

$$\text{Footing}_{\text{width}} := 6 \cdot \text{ft}$$

$$\text{Footing}_{\text{length}} := 8 \cdot \text{ft} \quad \text{Z direction}$$

$$\text{Column_Dia} := 36 \cdot \text{in}$$

$$\text{Soil_Depth_Above_Foot} := 5 \cdot \text{ft}$$

Be conservative on soil depth as this is helping us

$$\gamma_{\text{soil}} := 120 \cdot \text{pcf}$$

There are two ways to do this.

Enter in the Vertical soil load as

1) a distributed load in the longitudinal (z) direction

2) one concentrated force on the column in the DL.as DL and EV are in every case and have approximatley the same

Unfortunately and stupidly there is no way to enter in a pressure load at the top of the footing. I think i have done both depnding on what i needed.

I think here we can input the soil at one magnitude outside the column, and one magintuide (within) the column

$$\text{EV_End} := \text{Footing}_{\text{width}} \cdot \text{Soil_Depth_Above_Foot} \cdot \gamma_{\text{soil}} = 3.6 \cdot \text{klf}$$

$$\text{EV_Middle} := (\text{Footing}_{\text{width}} - \text{Column_Dia}) \cdot \text{Soil_Depth_Above_Foot} \cdot \gamma_{\text{soil}} = 1.8 \cdot \text{klf}$$

$$\text{EV_End} = 3.6 \cdot \text{klf} \quad \text{Z1_L}_1 := 0 \quad \text{Z2_L}_1 := \frac{0.5 \cdot (\text{Footing}_{\text{length}} - \text{Column_Dia})}{\text{Footing}_{\text{length}}} = 0.313$$

$$\text{EV_Middle} = 1.8 \cdot \text{klf} \quad \text{Z1_L}_2 := \text{Z2_L}_1 = 0.313 \quad \text{Z2_L}_2 := 1 - \text{Z1_L}_2 = 0.688$$

$$\text{EV_End} = 3.6 \cdot \text{klf} \quad \text{Z1_L}_3 := \text{Z2_L}_2 = 0.688 \quad \text{Z2_L}_3 := 1.0$$

How much it will reduce uplift by

$$EV_End \cdot (2 \cdot Z2_L1) \cdot Footing_{length} = 18 \cdot kip$$

$$EV_Middle \cdot (Z2_L2 - Z1_L2) \cdot Footing_{length} = 5.4 \cdot kip$$

$$Total_Force := EV_End \cdot (2 \cdot Z2_L1) \cdot Footing_{length} + EV_Middle \cdot (Z2_L2 - Z1_L2) \cdot Footing_{length} = 23.4 \cdot kip$$

$$Pressure := Total_Force \div (Footing_{width} \cdot Footing_{length}) = 0.488 \cdot ksf$$

LEFT PIER CAP

$$Neg_Press_L := -1.39 \cdot klf \quad \text{greatest service negative value @ corners 3 and 4}$$

$$Pos_Press_L := 7.65 \cdot klf \quad \text{least service positive value @ corners 1 and 2}$$

$$Inflection_Point := Footing_{length} \cdot \frac{Neg_Press_L}{(Pos_Press_L - Neg_Press_L)} = 1.23 \cdot ft$$

$$Centroid_Dist_Past_Inflection := \frac{2}{3} \cdot (Footing_{length} - Inflection_Point) = 4.513 \cdot ft$$

AASHTO 11.6.3.3

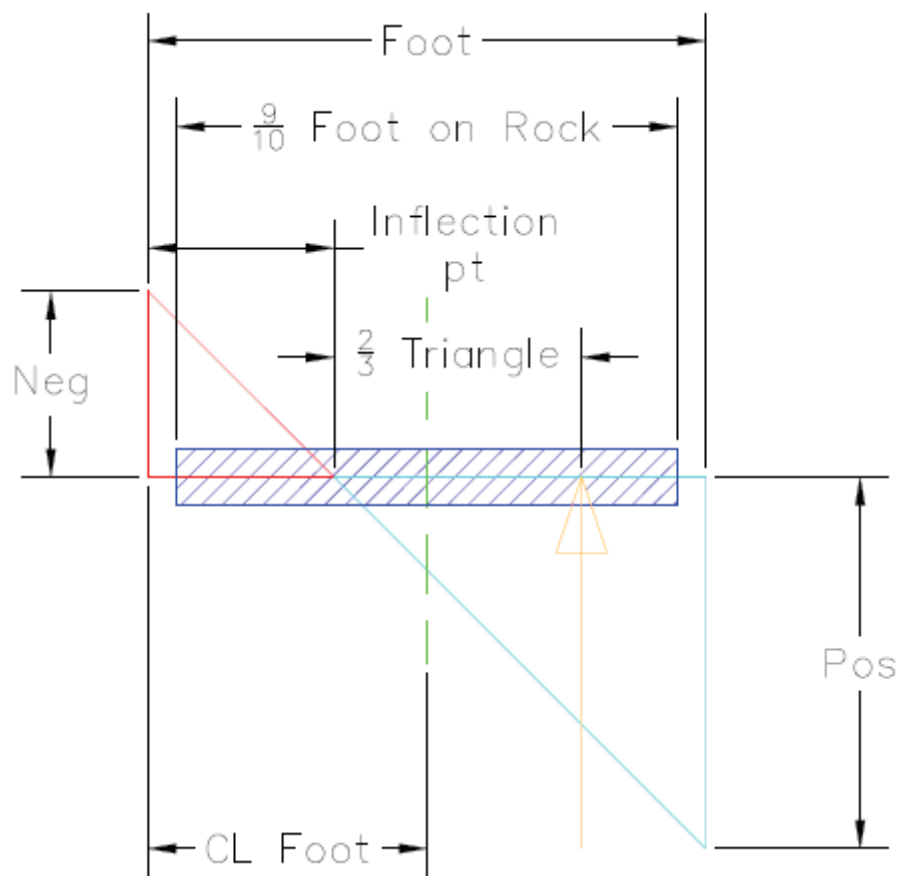
For Foundation on rock, the location of the resultant reaction forces shall be within the middle 9/10th of base

$$Ecc_Limit_1 := 0.05 \cdot Footing_{length} = 0.400 \cdot ft \quad Ecc_Limit_2 := 0.95 \cdot Footing_{length} = 7.600 \cdot ft$$

Eccentric limit is based on distance from edge of footing

$$Centroid_Location := Inflection_Point + Centroid_Dist_Past_Inflection = 5.743 \cdot ft$$

$$Eccen_Check := \text{if}(Ecc_Limit_1 \leq Centroid_Location \leq Ecc_Limit_2, "Okay", "No Good") = "Okay"$$



11.6.3.3—Eccentricity Limits

For foundations on soil, the location of the resultant of the reaction forces shall be within the middle two-thirds of the base width.

For foundations on rock, the location of the resultant of the reaction forces shall be within the middle nine-tenths of the base width.

11.6.3—Bearing Resistance and Stability at the Strength Limit State

RIGHT PIER CAP

$Neg_Press_R := -1.61 \cdot klf$ greatest service negative value @ corners 1 and 2

$Pos_Press_R := 6.11 \cdot klf$ least service positive value @ corners 3 and 4

$$Inflection_Point := \left| Footing_{length} \cdot \frac{Neg_Press_R}{(Pos_Press_R - Neg_Press_R)} \right| = 1.668 \cdot ft$$

$$Centroid_Dist_Past_Inflection := \frac{2}{3} \cdot (Footing_{length} - Inflection_Point) = 4.221 \cdot ft$$

AASHTO 11.6.3.3

For Foundation on rock, the location of the resultant reaction forces shall be within the middle 9/10th of base

$$Ecc_Limit_1 := 0.05 \cdot Footing_{length} = 0.400 \cdot ft \quad Ecc_Limit_2 := 0.95 \cdot Footing_{length} = 7.600 \cdot ft$$

Eccentric limit is based on distance from edge of footing

$$Centroid_Location := Inflection_Point + Centroid_Dist_Past_Inflection = 5.889 \cdot ft$$

$$Eccen_Check := \text{if} \left(Ecc_Limit_1 \leq Centroid_Location \leq Ecc_Limit_2, "Okay", "No Good" \right) = "Okay"$$