

MED - 224 - 17.66
SUM - 224 - 0.00

Note: This project has been subdivided into two parts, each of which requires grading, drainage, and pavement work and incidental performance as specified.

PART I = STA. 959+00 TO 1278+00 IN MED. CO.
PART II = STA. 1278+00 IN MED. CO. TO STA. 138+58 SUMMIT

LIMITED ACCESS

This improvement is especially designed for through traffic and has been declared a limited access highway or freeway by action of the Director of Highways in accordance with the provisions of Section 5511.02 of the Revised Code of Ohio.

The Standard Specifications of the State of Ohio, Department of Highways, including changes and Supplemental Specifications listed in the proposal shall govern this improvement.

I hereby approve these plans and declare that the making of this improvement will not require the closing to traffic of the highways except as noted on Sheet No 2 and that provisions for the maintenance and safety of traffic will be as set forth on the plans and estimates.

The Right-of-Way necessary for this improvement will be provided by the State of Ohio.

Approved W.C. Kautzman
Date: 2-26-57 Division Deputy Director

Approved: C. H. Makuver
Date: 5-27-57 Deputy Director of Planning and Programming

Approved: D. H. Overman mce
Date: 5-16-57 Engineer of Bridges

Approved: R.E. Shultz
Date: 5-17-57 Engineer of Location and Design.

Approved: P.E. Masket
Date: 5-17-57 Deputy Director of Design and Construction

Approved: _____
Date: _____ First Assistant Director

Approved: George J. Thormann
Date: 5/22/57 Acting Director of Highways

Approved: J. W. Wier, Jr.
Date: 1-22-57 Steepleton, McDormell, & Barber, Associated Engineers
and Architect

SEP 7 1961
GROUND PHOTOGRAPH

Sheet 5- Revised 7-12-57- G.H.J.

SUPPLEMENTAL SPECIFICATIONS

B-119	Rev.	6-30
E-101		1-1-5
5		6-8-5
18	Rev.	2-6-5
M-206.6(b)		5-25
S-114		8-30

DEPARTMENT OF COMMERCE
BUREAU OF PUBLIC ROADS

APPROVED:

DISTRICT ENGINEER

DATE _____

PLANS BY STEPLETON, McDONNELL, & BARBER - TOLEDO, OHIO

Reviewed and Approved: H. E. Schmitt
Date 3/1/57 Engineer of Traffic

WADSWORTH AND GUILFORD TOWNSHIPS
& CITY OF WADSWORTH, MEDINA COUNTY
AND NORTON TOWNSHIP
SUMMIT COUNTY

CONVENTIONAL SIGNS

COUNTY LINE
 TOWNSHIP LINE
 SECTION LINE
 CITY OR VILLAGE LINE
 CENTER LINE
 PROPERTY LINE
 FENCE LINE
 GUARD RAIL
 POLE LINES
 RAILROAD
 DRAINAGE PIPES

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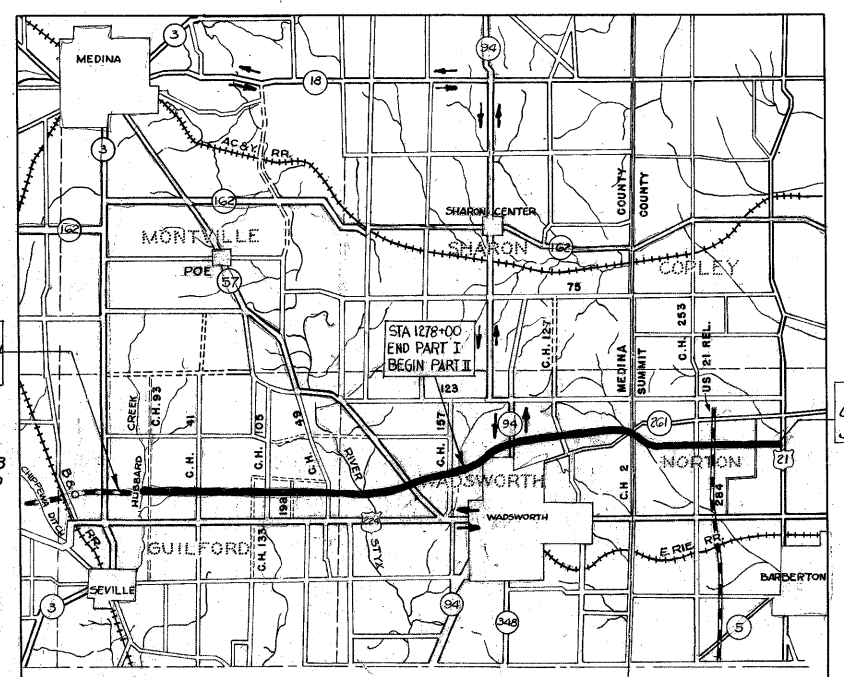
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LINE DATA

BEGIN PT. I PROJ. F-458(14) 959+00.00 (MEDINA CO.)
 END PT. I STA. 1270+00.00 NET LENGTH PROJ. PT. I = 31,900.00 Lin.Ft. or 6.042 Miles
 BEGIN PT. II PROJ. F-458(14) 1270+00.00
 TO STA. 1299+80.00 (WADSWORTH CORRLINE) = 2130.00 Lin.Ft.
 TO STA. 1302+16.85 (WADSWORTH CORRLINE) = 236.85 " "
 TO STA. 1455+70.03 (MEDINA-SUMMIT CO. LINE) = 15,353.18 " "
 STATION EQUATION: 1455+70.03 MEDINA CO. = 0+00.00 SUMMIT CO.
 END PT. II PROJECT F-458(14) STA. 1367+35.00 (SUMMIT CO.) = 13,635.00 " "
 NET LENGTH PROJECT F-458(14) = 31,405.03 Lin.Ft. or 5.941 Miles
 TOTAL LENGTH PROJECT F-458(14) = 63,305.03 Lin.Ft. or 11.989 Miles
 WORK PT. I F-458(14):
 APPR. C.H. 93 STA. 7+75 to 15+55 (-24') = 756 Lin.Ft.
 APPR. S.R. 57 STA. 2+80 to 28+25 = 2345 " "
 APPR. C.H. 157 STA. 0+75 to 8+25 (-88') = 662 " "
 TOTAL WORK PT. I = 31900 + 3363 = 35263 Lin.Ft. or 6.792 Miles
 WORK PT. II F-458(14): APPR. STA. 1367+35 to 138+58 = 223
 APPR. S.R. 261 STA. 6+40 to 13+30 = 690 Lin.Ft.
 APPR. C.H. 2 STA. 8+49 to 15+75 (-111) = 615 " "
 APPR. C.H. 253 STA. 8+75 to 14+25 (-88') = 462 " "
 APPR. U.S. 214 (R.Loc.) STA. 172+47 to 208+00 = 3553 " "
 APPR. S.R. 94 STA. 2+50 to 18+00 = 1550 " "

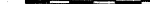
$$\text{TOTAL WORK PT. II} = 31,405.03 + 7093.00 = 38498.03 \text{ Lin. Ft. or } 7.291 \text{ Mi}$$

TOTAL WORK F-458(14)
= 35063.00 Lin.Ft + 38493.03 Lin.Ft
= 74361.03 Lin.Ft or 14.083 Miles



Delivery Point: Wodsworth. Average Haul: 4 Miles

LOCATION PLAN


 Scale of Miles

Detour


Portion to be improved 

State Roads 

County Roads 

Road under construction 

SCA/FS

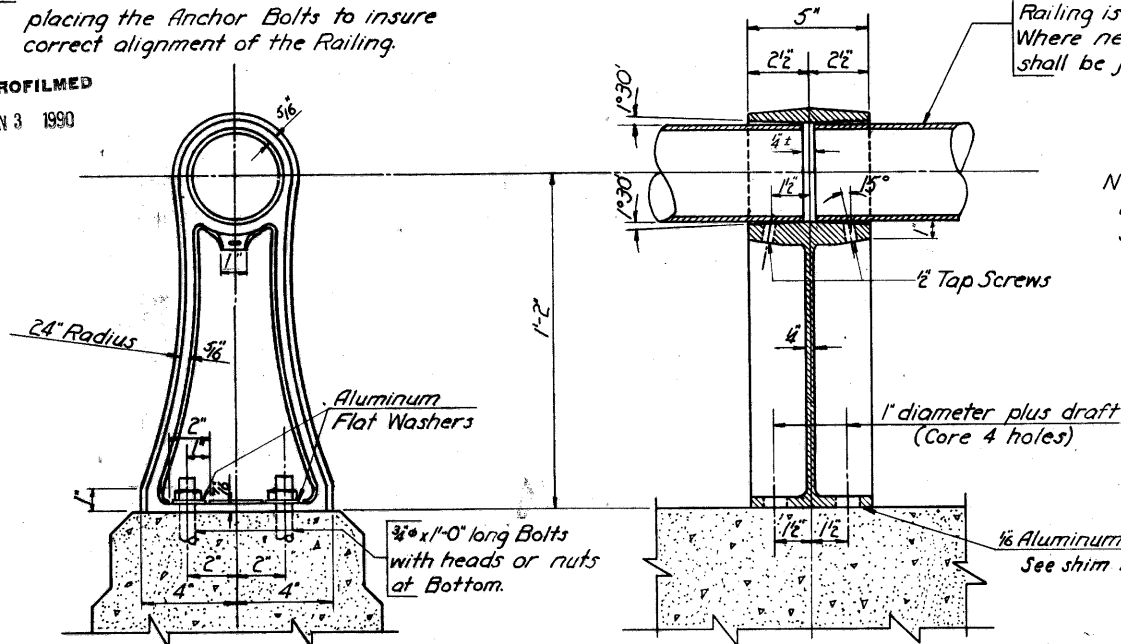
PLAN: $1"=50'$
 PROFILE: Hor. $1"=50'$
 Vert. $1"=5'$ & $1"=10'$
 CROSS SECTIONS: $1"=10'$

STANDARD CONSTRUCTION DRAWINGS

STANDARD CONSTRUCTION DRAWINGS					
DRAWING NO.	DATE	DRAWING NO.	DATE	DRAWING NO.	DATE
RB-1-55	3-1-55	CSB-1-55 (Sheet 2)	12-3-56	P-1-54	12-1-55
A-1-54	12-1-54	CS-1-54 (2 Sheets)	7-16-56		
L-1	4-1-50	S-27 PC-3	2-20-55	I-15 N ^o 2A	7-2-55
L-3	4-1-50	S-27 PC-4	1-4-54	I-21-23	8-1-55
L-3A	4-1-50	I-12, 3, 4 & 5	2-20-55	I-8 MH N ^o 1A	1-3-55
RI-1	1-3-55	I-8 C.B. 2, 2A & B	8-1-56	G-7-07	6-1-55
T-35	1-2-56	I-8 C.B. N ^o 3	5-1-52	B-1-50, 70-71 E N ^o 1	10-1-44
B-7-11R	3-2-53	I-8 C.B. N ^o 6	5-1-52	I-8 MH N ^o 3	5-1-55
LJ N ^o 1	7-1-55	I-8 C.B. N ^o 7	5-1-52	OS-1	12-11-55
TJ	5-1-56	I-8 M.H. N ^o 1	5-1-52	OS-2	12-11-55
DR-1	1-3-55	I-12	7-1-54	L-2	4-1-55
AS-1-54	12-1-54	I-14 G	1-22-52	RD-1	7-2-55
S-27 PC-1	5-1-52	I-15 N ^o 1	8-1-55	RP-2	9-14-55
I-8 C.B. N ^o 2, 3 & 2-4	5-1-52	I-15 N ^o 2	12-1-54	RP-3	9-14-55
SD-53	7-21-53				

NOTE: Extreme care shall be exercised in placing the Anchor Bolts to insure correct alignment of the Railing.

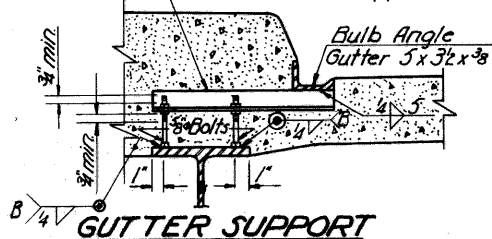
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JAN 3 1990



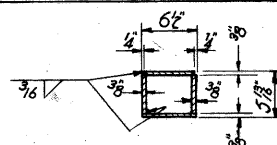
NOTE: Reference shall be made to Supplemental Specification No. S-114, Aluminum for Bridge Railing, dated August 30, 1953.

ALUMINUM RAILING DETAIL

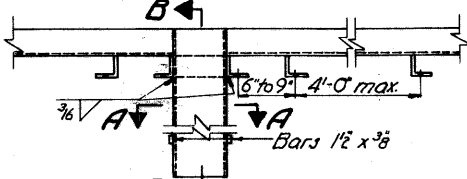
Gutter Support is 2x2x1/4 spaced not more than 4'-0" c.c. between Scuppers. Slotted holes 1/2" x 1/2".



GUTTER SUPPORT



SECTION A-A



REAR ELEVATION

GUTTER AND SCUPPER NOTES

- See General Plan for spacing of scuppers. Scuppers must be spaced to clear crossframes.
- Milled joints will be permitted in bulb angles, but individual lengths shall be made as long as practicable.
- Supports shall be placed 6" to 9" on each side of joints.
- Gutters shall be accurately adjusted for alignment and grade, with allowance for dead load deflection, before concrete is placed.
- Welding shall be Class "A" except as shown. Any welds shown as field welds may, at the option of the Contractor, be made in the shop.
- Class "B" welds are indicated thus B.

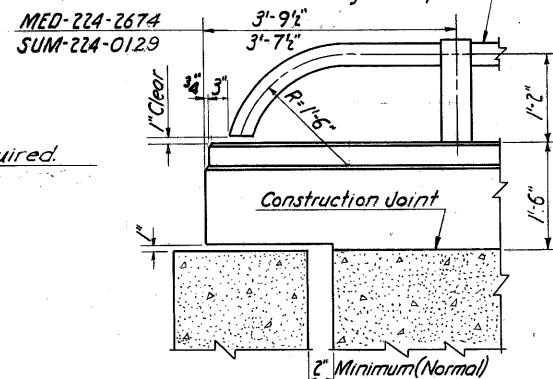
File No 220-M

4" O.D. x 3/8 wall aluminum tube. Railing is generally continuous. Where necessary railing tube shall be joined as shown.

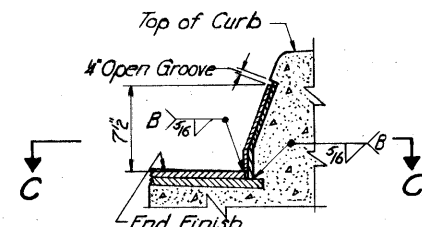
NOTES: Shims shall be used where necessary for vertical alignment. They shall be of aluminum alloy, 2 1/2 x 1/2 x 0-8" slotted for anchor bolts to permit insertion after posts are in place.

The tubes shall be closed with a welded-in aluminum plate (1/4 min. thickness) at ends adjacent to concrete end posts.

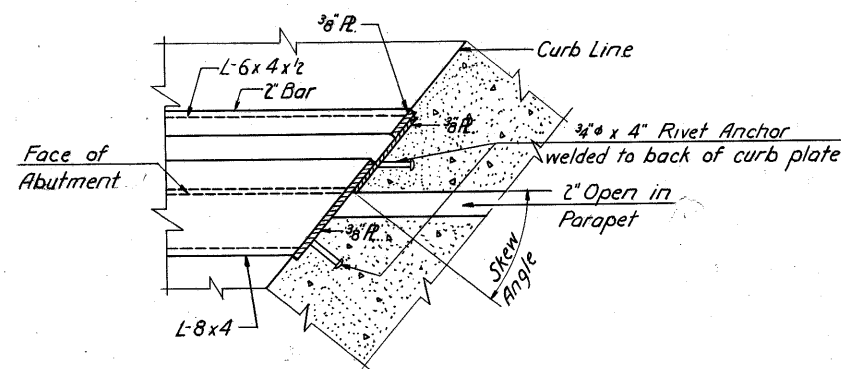
4" O.D. x 3/8 wall aluminum tube. Railing shall be continuous through first post.



SECTION E-E

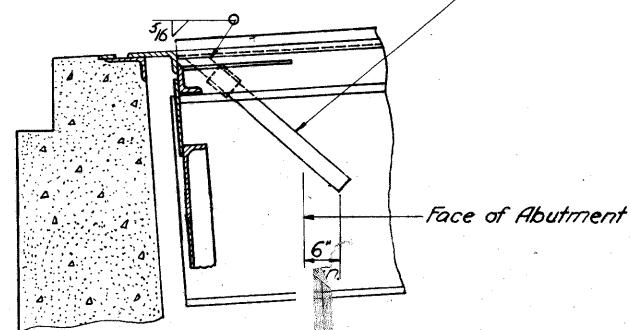


SPLASH GUARD DETAIL



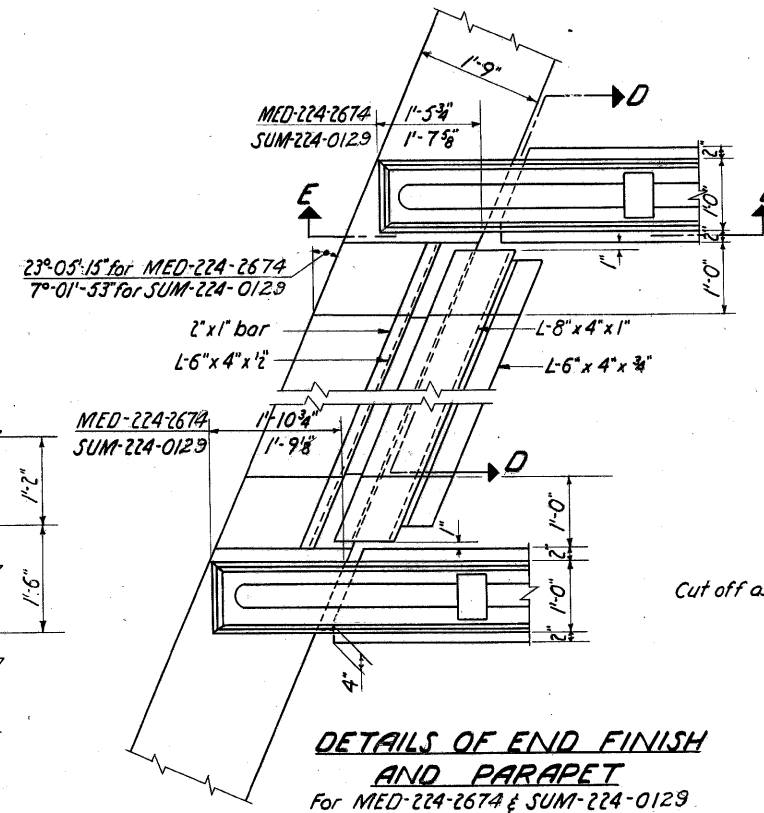
SECTION C-C

3" Diameter Standard Steel Pipe. Weld stub end full perimeter to Bulb L and provide coupling.



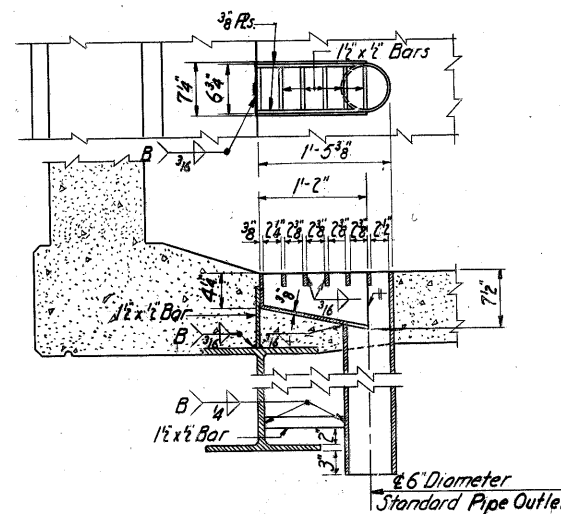
SCUPPER DETAIL AT LO END FINISH

Locate at ends of bridge where grade slops down to end finish.



DETAILS OF END FINISH AND PARAPET

For MED-224-2674 & SUM-224-0129

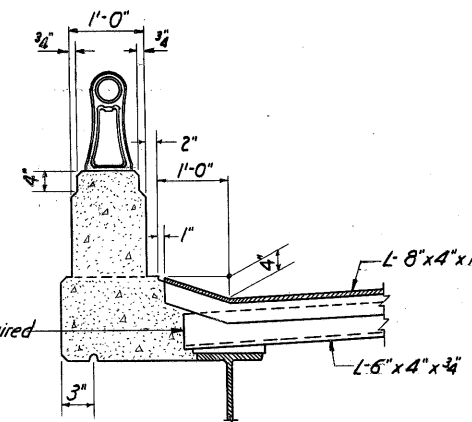


SCUPPER DETAIL

For MED-224-2674 & SUM-224-0129

FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

MED-224-17.66
SUM-224-0.00

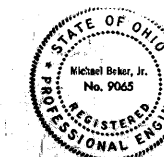


SECTION D-D

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

COMMON DETAILS

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE
		P.V.C.D.			4-8-57



MEDINA COUNTY
MED-224-17.66
SUM-224-0.00

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NOTE: Foundation design and foundation quantities are based on a study of rod soundings (and/or soil sampling soundings) made at the site. This sounding information may be inspected in the office of the Bureau of Bridges in Columbus or in the Division office, but the State assumes no responsibility for the accuracy thereof.

SECTION A-A

Place Dumped Rock Fill up to Elev. 1038 against Fill Slopes

End Approach Slope	St. 078 + 20.75
--------------------	-----------------

Begin Approach Sl	
Sta 979+19.75	

Profile Grade Point—

Sta. 979+44.75

Sta. 979+44.75

Survey & 2-Lane Constr. 381 U.S. 221 (Prop.) 382

EXISTING BRIDGE DATA

Upstream on C.H. 43 (250' So. of C.H. 118)
Span: 36'
Type: Steel Beam on Stone Abut.
Corr. Met. Deck
Roadway: 15'
Bottom of Deck to F/L = 9'

Upstream on C.H. 118
Span: 30'
Type: Conc. Slab Deck on Conc. Abut.
Roadway: 18'
Bottom of Deck to F/L = 8'

EXISTING BRIDGE DATA

Bridge No. ME-224-176 over Hubbard
Creek on U.S. 824 Downstream
TYPE: Concrete Slab
SPAN: 2 @ 26'
ROADWAY: 34'
OUT to OUT DECK: 38'
BOTTOM OF DECK TO FIL = 10'

PROPOSED STRUCTURE

TYPE- Continuous reinforced concrete
slab with capped pile abutments
and cap and column piers.
SPAN- 30'- 37.5' - 30' c/c bearings.
ROADWAY- 44'-0" $\frac{1}{2}$ of guard rails.
LOAD-FREQUENCY CF= 2000 (51)
SKEW- None
WEARING SURFACE- 1" Monolithic
Concrete
APPROACH SLAB- AS-1- 54' (25' long)
ALIGNMENT- Tangent

Drainage Area = 6.04 Sq. Mi.

STEPLETON, McDONNELL & BARBER
ASSOCIATED ENGINEERS & ARCHITECTS
TOLEDO 2, OHIO

SITE PLAN
BRIDGE No. MED-224-1802
Over Hubbard Creek
MEDINA COUNTY 978 + 20.75
STA. 979 + 19.75

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
	J.W.	J.C.P.				

DESIGN ENGINEER - MICHAEL BAKER JR

MICROFILMED

JAN 3, 1990

Bridge Limits = 990'

Slab thickness = 16" which includes 1" monolithic wearing surface.



FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

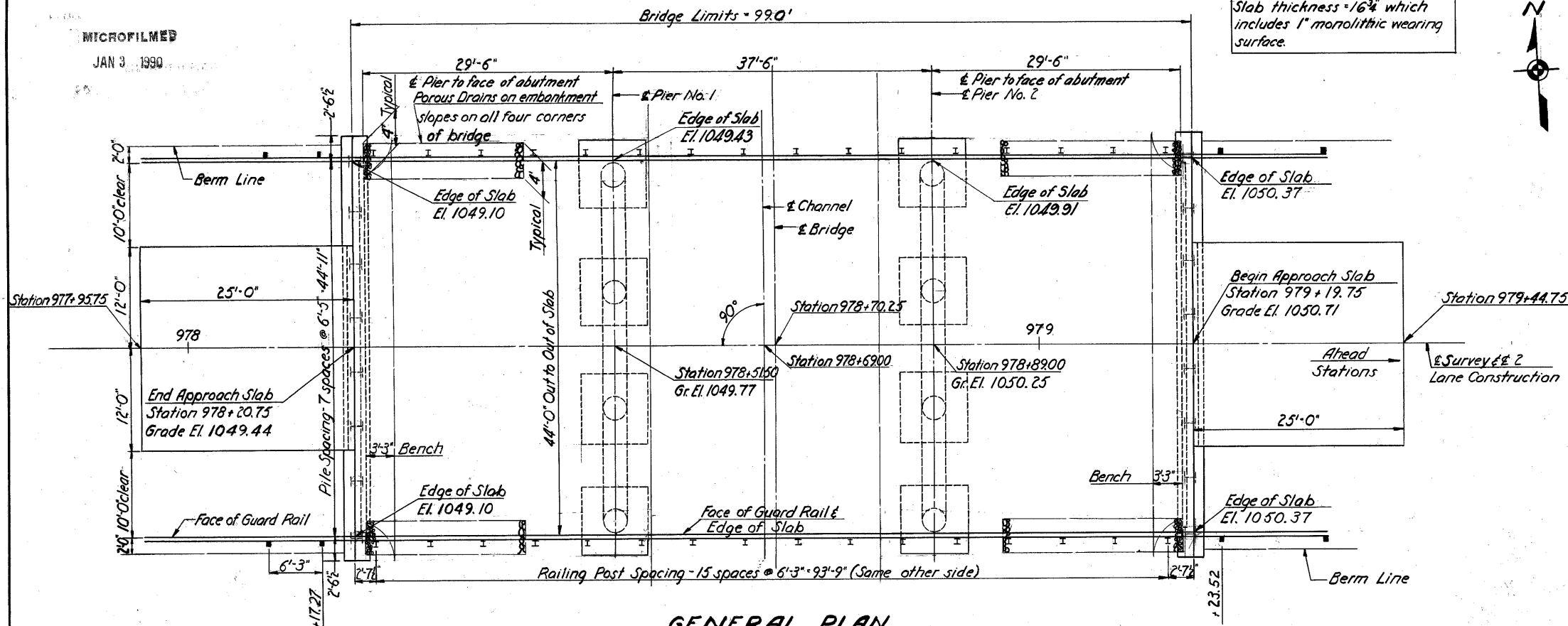
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MEDINA COUNTY
MED-224-17.66
SUM-224-0.00

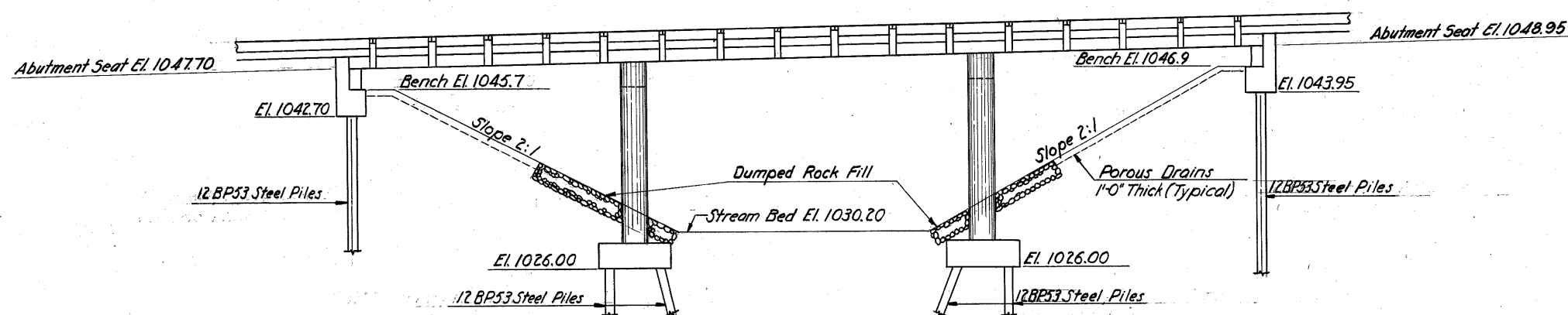
GENERAL NOTES

- Reference shall be made to the following Standard Drawings:
Slab CS-1-54 Date 7-1-54 Revised 7-16-56
Abutments A-1-54 Date 7-1-54 Revised 12-1-54
- Design Specifications: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 10-1-51, together with revisions thereof dated 7-15-52, 4-1-54 and 2-1-55.
- Excavation quantity includes the removal of fill material between the top of the earth bench and the bottom of the abutment.
- Galvanizing of all members which are specified to be galvanized shall be as called for in Section M-7.4(d).
- Porous drains, extending from face of abutment to Elevation 1038.0 shall be provided at all four corners of the bridge. The drains shall be 4 feet wide and one foot thick.
- Gravel, if used as the coarse aggregate, shall be according to Section M-3.93 for Class "C" Concrete in the superstructure. Gravel meeting the requirements of Section M-3.93 also may be used for other concrete in this structure.
- Piles shall be driven to firm contact with dense clay, which shall be considered as attained when the capacity according to the formula in Section S-18.05 is at least 40 tons per pile for the pier piles and 35 tons for the abutment piles if a 7000 ft. lb. hammer is used, or 30 tons for the pier piles and 27 tons for the abutment piles if a hammer of 11,000 ft. lb. or greater energy is used and if the length of penetration is approximately equal to the estimated length according to the bridge foundation investigation report. If the energy rating of the hammer is between these values the required formula capacity shall be determined by interpolation. (The design load is 30 tons per pile for the pier piles and 27 tons for the abutment piles.) The length of penetration of every pile shall be at least 80% of the estimated average length of penetration of the piles in the pertinent pier or abutment as indicated on the plans unless a lesser penetration is approved by the Director.

GENERAL PLAN



ELEVATION



REINFORCING STEEL LIST

Superstructure					Abutments				
Mark	No.	Length	Weight	Type	Mark	No.	Length	Weight	Type
F1067	80	27'-2"	9353	Str.	R1001	16	22'-11"	1578	Str.
G1067	40	12'-11"	2224	Str.	R801	16	26'-1"	1114	Str.
H1067	38	8'-10"	1444	Str.	R501	16	25'-7"	427	Str.
A967	132	34'-10"	15632	Str.	R502	156	6'-7"	1070	Bt.
B967	44	24'-11"	3728	Bt.	R503	8	22'-1"	184	Str.
C967	42	21'-11"	3130	Bt.	R504	24	5'-4"	133	Str.
D967	22	23'-4"	1745	Str.	R505	32	7'-11"	264	Bt.
E967	21	18'-2"	1297	Str.	R506	8	12'-6"	104	Str.
M701	91	43'-6"	8091	Str.	R507	16	4'-11"	82	Str.
J601	40	18'-4"	1101	Str.	R508	28	6'-8"	195	Bt.
K601	20	15'-4"	460	Str.	R509	28	8'-5"	246	Bt.
N601	63	43'-6"	4116	Str.	R401	64	5'-5"	232	Bt.
Total Weights for Superstructure = 52,321					Total Weight for Abutments = 5629*				

REPLACEMENT BAR SCHEDULE

Mark	No.	Size	Length	Type	Weight
RE401	1	4	5'-3"	Str.	
RE502	1	5	5'-7"	Str.	
RE603	1	6	5'-11"	Str.	
RE704	1	7	6'-2"	Str.	
RE805	1	8	6'-6"	Str.	
RE906	2	9	6'-10"	Str.	
RE1007	1	10	7'-1"	Str.	

If reinforcing bars are fabricated from stock which has previously been tested and approved by the Ohio Highway Testing Laboratory, test samples as provided in Sec. S-4.02 need not be furnished and replacement bars will not be required.

ESTIMATED QUANTITIES

Item	Total	Unit	Description	Superstr.	Piers	Abutments	General
E-2	200	Cu. Yds.	Unclassified Excavation		140	60	
E-3	640	Cu. Yds.	Channel Excavation				640
S-1	228	Cu. Yds.	Class "C" Concrete, Superstructure	228			
S-1	52	Cu. Yds.	Class "C" Concrete, Pier Caps and Columns		52		
S-1	51	Cu. Yds.	Class "E" Concrete, Abutments			51	
S-1	44	Cu. Yds.	Class "E" Concrete, Pier Footings		44		
S-4	72,268	Lbs.	Reinforcing Steel	52,321	14,318	5,629	
S-14	198	Lin. Ft.	Railing (Type I-15.13 with galvanized Steel Posts & Bolts)	198			
S-16	Lump	Sum	First Test Pile				
S-18	1290	Lin. Ft.	Steel Piles, 12BP53		780	510	
S-29	13		Porous Drains on Embankment Slopes				13
S-29	22		Porous Backfill			22	
I-10	195	Cu. Yds.	Dumped Rock Fill				195
E-2	Lump	Sum	Cofferdams, Cribbs, and Sheet piling				

MICHAEL BAKER, JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

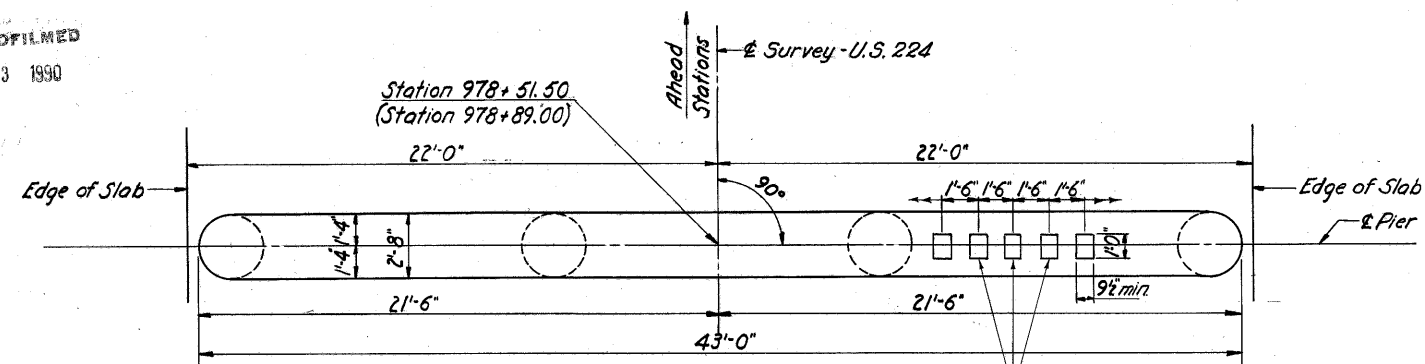
GENERAL PLAN & ELEVATION

BRIDGE NO. MED. 224-18Q2
OVER HUBBARD CREEK

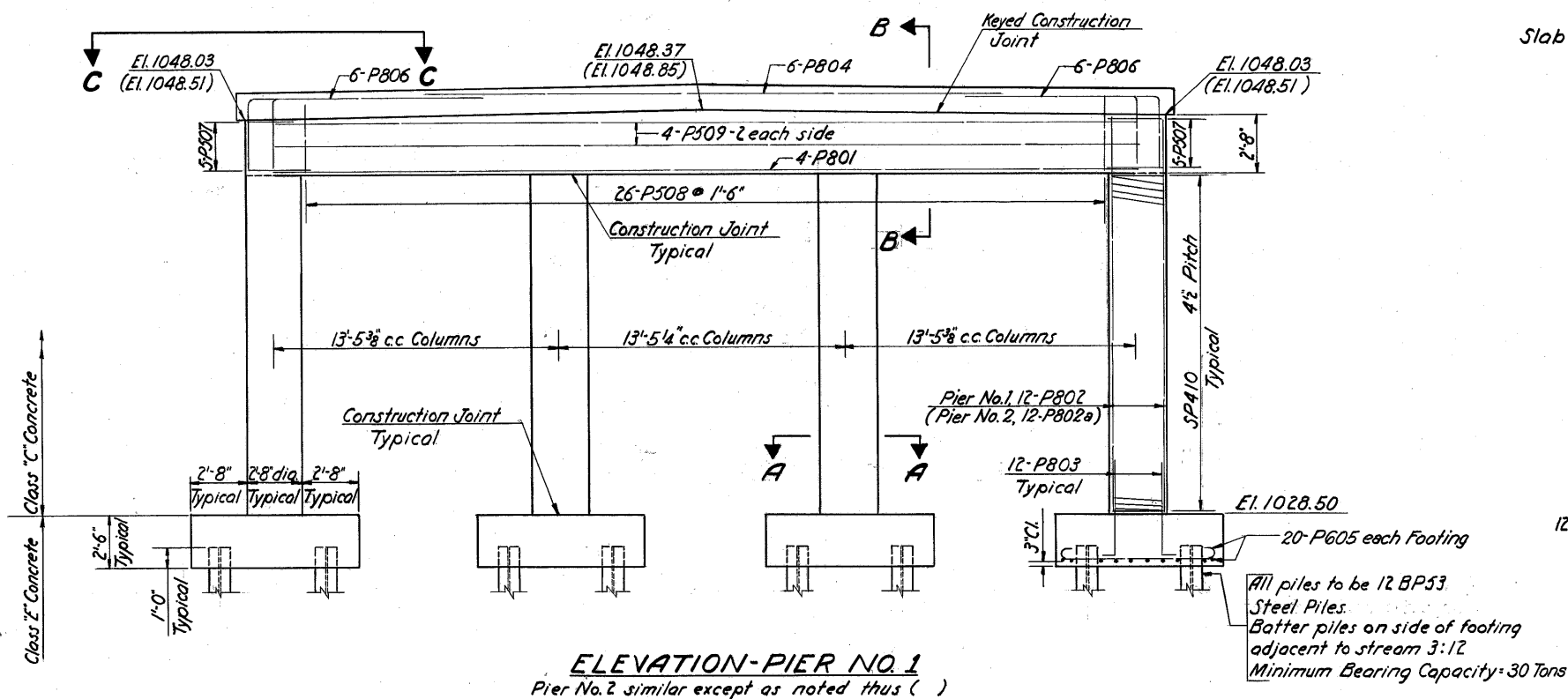
MEDINA COUNTY		DESIGNED		DRAWN		CHECKED		REVIEWED	
FWL	FWL	P.D.	FK						

DESIGN ENGINEER-MICHAEL BAKER JR.

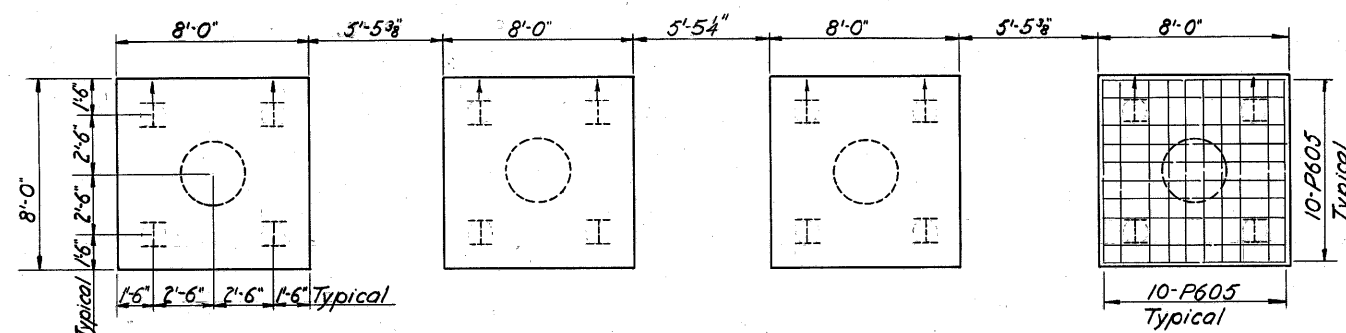
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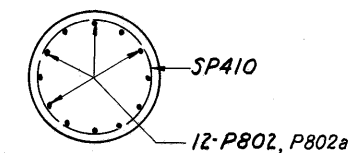
CAP PLAN-PIER NO. 1
Pier No. 2 similar except as noted thus ()



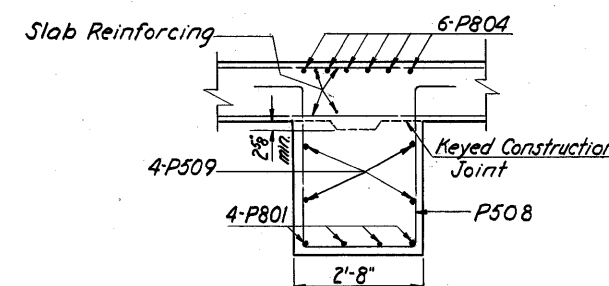
ELEVATION-PIER NO. 1
Pier No. 2 similar except as noted thus ()



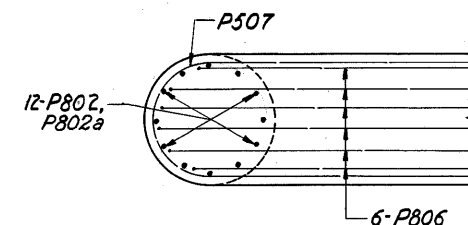
FOOTING PLAN



SECTION A-A



SECTION B-B



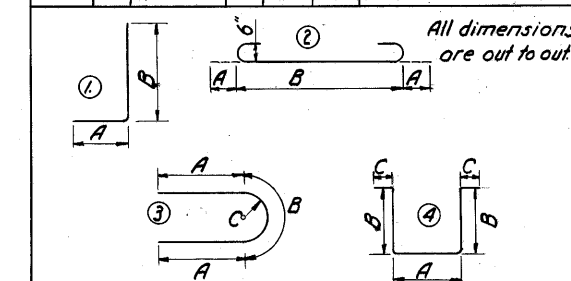
SECTION C-C

FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

MEDINA COUNTY
MED-224-17.66
SUM-224-0.00

BAR SCHEDULE FOR TWO PIERS

Mark No.	Length	Size	Type	A	B	C	Weight
P801	8	41'-0"	8	Str.			876
P802	48	19'-4"	8	Str.			2477
P803	96	5'-2"	8	1	8"	4'-8"	1325
P804	12	28'-0"	8	Str.			897
P805	160	8'-10"	6	2	8"	7'-6"	2122
P806	24	13'-6"	8	1	3'-9"	10'-0"	865
P507	20	6'-10"	5	3	1'-7"	3'-8"	142
P508	52	10'-8"	5	4	2'-4"	3'-8"	579
P509	8	41'-0"	5	Str.			342
P802a	48	19'-10"	8	Str.			2541



SPIRAL BARS FOR TWO PIERS

Mark	No.	Length	Pitch	Wool Turns	Core Diam.	Size	Weight
SP410	8	16'-9"	4 1/2"	48	28"	4	2,152

Total Weight = 14,318 #

Total Weight = 14,318 #

NOTES

- Bar size is indicated in the bar mark. The first digit where three digits are used indicate the bar size number. For example, P805 is a No. 8 size bar.
- Clearance of Reinforcing Steel from face of concrete shall be 2" unless otherwise noted.
- Falsework for pier cap shall remain in place until deck slab is poured and cured.

SPIRAL NOTES

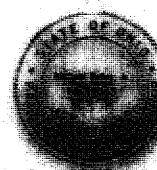
- The "length" shown in the steel list for the spiral bars is the distance from the top of the footing to the bottom of the pier cap.
- The "No. of Turns" shown in the steel list for the spiral bars is the "length" divided by the pitch, plus 3 turns (total number of closed coils), expressed as the nearest whole number.
- Spiral reinforcing bars shall not have deformations, but shall in other respects conform to Item S-4. 1/2 closed coils shall be provided at the ends of each spiral unit.
- Three Steel Channel, tee or angle spacers, weighing approximately 0.68 lbs. per lin. ft., shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil.
- The number of pounds of these spacers, based on 0.68 lbs. per lin. ft., will be paid for as reinforcing steel and is included in the tabulated quantity of spiral bars.

MICHAEL BAKER, JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

PIERS

BRIDGE NO. MED. 224-1802
OVER HUBBARD CREEK

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED-DATE	REVISED
F.W.L.	F.W.L.	P.D.	F.W.L.		



DESIGN ENGINEER-MICHAEL BAKER JR.