

ITEM 623 - CONSTRUCTION LAYOUT STAKES AND SURVEYING, AS PER PLAN

IN ADDITION TO THE REQUIREMENTS OF ITEM 623 CONSTRUCTION LAYOUT STAKES AND SURVEYING, THE CONTRACTOR SHALL PROVIDE THE FOLLOWING INFORMATION TO THE DEPARTMENT:

THE CONTRACTOR SHALL PROVIDE AS-BUILT DATA FOR THE SPECIFIED COMPLETED CONSTRUCTION ITEMS IN OHIO STATE PLANE COORDINATES (GRID). THE CONSTRUCTION ITEMS SHALL BE LOCATED AS PER THE SURVEY FEATURE CODE LIST FOUND ON THE OHIO DEPARTMENT OF TRANSPORTATION OFFICE OF CADD & MAPPING SERVICES WEBSITE. A CD CONTAINING A COMMA DELIMITED ASCII FILE AND A SURVEYOR'S CERTIFICATION SHALL BE DELIVERED TO THE PROJECT ENGINEER AFTER ALL INFORMATION HAS BEEN COLLECTED. THE ASCII FILE SHALL INCLUDE A HEADER CONTAINING NAME OF SURVEYOR, DATE(S) OF COLLECTION, HORIZONTAL DATUM (I.E. NAD83 (2011), OHIO STATE PLANE COORDINATE SYSTEM NORTH OR SOUTH), VERTICAL DATUM (I.E. NAVD 88, GEOID12A) AND METHOD OF COLLECTION (I.E. OHIO VRS, GPS RTK, TOTAL STATION, ETC.) AND BE IN A TABLE FORM AS FOLLOWS:

POINT NUMBER, NORTHING, EASTING, ELEVATION, FEATURE CODE, DESCRIPTION

BELOW IS A LIST OF THE ITEMS THE CONTRACTOR IS REQUIRED TO **PROVIDE FOR THE PROJECT:**

- RIGHT-OF-WAY FENCE (POINTS AT ALL CHANGES IN DIRECTION)
- LIGHT POLES AND LIGHTING PULLBOXES
- BARRIER (GUARDRAIL, CONCRETE OR CABLE)
- BMP'S (SEE PROJECT SITE PLAN FOR INFO)
- CULVERTS (INLET INVERT, OUTLET INVERT, TYPE, AND SIZE)
- STORM SEWER OUTLETS (OUTLET INVERT, TYPE, AND SIZE)
- CATCH BASINS, MANHOLES, AND INLETS
- UNDERDRAIN OUTLETS
- SIGNS (WITH DESCRIPTIONS)
- TRAFFIC SIGNAL POLES, CONTROLLER LOCATION, AND SIGNAL PULLBOXES

THE ABOVE ITEMS SHALL BE COLLECTED USING SURVEY GRADE EQUIPMENT MEETING THE REQUIREMENTS OF SECTION 400 IN THE OHIO **DEPARTMENT OF TRANSPORTATION SURVEY & MAPPING SPECIFICATIONS** MANUAL

ALL COST ASSOCIATED WITH OBTAINING THE INFORMATION LISTED ABOVE SHALL BE INCLUDED IN THE LUMP SUM BID FOR ITEM 623 CONSTRUCTION LAYOUT STAKES AND SURVEYING, AS PER PLAN.

ALL MATERIALS, LABOR, AND EQUIPMENT RELATED TO MAINTAINING USABLE CONTROL POINTS AND ASSOCIATED REPORTS SHALL BE INCLUDED IN ITEM 623 CONSTRUCTION LAYOUT STAKES AND SURVEYING, AS PER PLAN. (SEE NOTE ON SHEET P.23 FOR SURVEY PARAMETERS)

CONSTRUCTION NOISE

ACTIVITIES AND LAND USE ADJACENT TO THIS PROJECT MAY BE AFFECTED BY CONSTRUCTION NOISE. IN ORDER TO MINIMIZE ANY ADVERSE CONSTRUCTION NOISE IMPACTS, DO NOT OPERATE POWER-OPERATED CONSTRUCTION-TYPE DEVICES BETWEEN THE HOURS OF 11:00PM AND 7:00AM. IN ADDITION, DO NOT OPERATE AT ANY TIME ANY DEVICE IN SUCH A MANNER THAT THE NOISE CREATED SUBSTANTIALLY EXCEEDS THE NOISE CUSTOMARILY AND NECESSARILY ATTENDANT TO THE REASONABLE AND EFFICIENT PERFORMANCE OF SUCH EQUIPMENT.

POST CONSTRUCTION STORM WATER TREATMENT

THIS PLAN UTILIZES STRUCTURAL BEST MANAGEMENT PRACTICES (BMP'S) FOR POST CONSTRUCTION STORM WATER TREATMENT.

VEGETATED FILTER STRIP

THIS PLAN UTILIZES VEGETATED FILTER STRIPS FOR POST CONSTRUCTION STORM WATER TREATMENT. PLACE EITHER ITEM 660 SODDING OR ITEM 659 SEEDING AND MULCHING WITH A 4-INCH LIFT OF TOPSOIL AND ITEM 670, SLOPE EROSION PROTECTION TO ALL DISTURBED AREAS DESIGNATED AS VEGETATED FILTER STRIPS, THE EDGE OF SHOULDER, AND THE FORESLOPE AS SPECIFIED IN THE PLANS.

VEGETATED BIOFILTER

THIS PLAN UTILIZES VEGETATED BIOFILTERS FOR POST CONSTRUCTION STORM WATER TREATMENT. PLACE EITHER ITEM 660 SODDING OR ITEM 659 SEEDING AND MULCHING WITH A 4-INCH LIFT OF TOPSOIL AS SHOWN IN THE PLANS TO ANY DISTURBED AREA ON THE SHOULDER AND FORESLOPE DRAINING TO A VEGETATED BIOFILTER. THE DITCH FOR EACH VEGETATED BIOFILTER SHALL BE TRAPEZOIDAL, AS SHOWN IN THE PLAN CROSS SECTIONS. PROVIDE ITEM 670 AS SPECIFIED IN THE PLANS.

PIPE CONNECTIONS TO CORRUGATED METAL STRUCTURES

PROVIDE CONNECTIONS OF PROPOSED LONGITUDINAL DRAINAGE TO CORRUGATED METAL STRUCTURES BY MEANS OF A SHOP FABRICATED OR FIELD WELDED STUB ON THE STRUCTURE. FURNISH A STUB MEETING THE REQUIREMENTS OF 707 WITH A MINIMUM LENGTH OF 2 FEET AND A MINIMUM WALL THICKNESS OF 0.064 INCHES.

THE LOCATION AND ELEVATION OF THE STUB ARE TO BE CONSIDERED APPROXIMATE AND MAY BE ADJUSTED BY THE ENGINEER TO AVOID CUTTING THROUGH JOINTS IN THE STRUCTURE.

THOROUGHLY CLEAN AND REGALVANIZE OR OTHERWISE SUITABLY REPAIR THE FIELD WELDED JOINT, IF USED. MEET WELDING REQUIREMENTS OF 513.21.

PROVIDE A MASONRY COLLAR PER STANDARD CONSTRUCTION DRAWING DM-1.1. TO CONNECT THE LONGITUDINAL DRAINAGE TO THE STUB, WHEN PIPE OTHER THAN CORRUGATED METAL IS USED FOR THE LONGITUDINAL DRAINAGE.

PAYMENT FOR CUTTING INTO THE STRUCTURE AND PROVIDING THE CONNECTION DESCRIBED, IS INCLUDED IN THE CONTRACT PRICE FOR ITEM 611 OR 522.

ENDANGERED BAT HABITAT REMOVAL

THIS PROJECT IS LOCATED WITHIN THE KNOWN HABITAT RANGES OF THE FEDERALLY LISTED AND PROTECTED INDIANA BAT, AND NORTHERN LONG-EARED BAT. NO TREES SHALL BE REMOVED UNDER THIS PROJECT FROM APRIL 1 THROUGH SEPTEMBER 30. ALL NECESSARY TREE REMOVAL SHALL OCCUR FROM OCTOBER 1 THROUGH MARCH 31. THIS REQUIREMENT IS NECESSARY TO AVOID AND MINIMIZE IMPACTS TO THESE SPECIES AS REQUIRED BY THE ENDANGERED SPECIES ACT (ESA). FOR THE PURPOSES OF THIS NOTE, A TREE IS DEFINED AS: A LIVE, DYING, OR DEAD WOODY PLANT, WITH A TRUNK 3 INCHES OR GREATER IN DIAMETER AT A HEIGHT OF 4.5 FEET ABOVE THE GROUND SURFACE, AND WITH A MINIMUM HEIGHT OF 13 FEET.

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ITEM 611 - CONDUIT UNDER RAILROAD

THE STATE SHALL PAY TO THE RAIL COMPANY ALL COSTS FOR WATCHMEN OR FLAGGERS DEEMED NECESSARY BY THE RAIL COMPANY, OR OCCASIONED BY THE OPERATIONS OF THE CONTRACTOR, OR ANY SUB-CONTRACTOR, IN CARRYING FORWARD THE INSTALLATION OF PIPE OR CONDUIT UNDER THE RAILROAD PER THE PLAN. THE COSTS FOR WATCHMEN OR FLAGGERS REQUIRED BY AN ALTERNATE METHOD OF INSTALLATION SHALL BE PAID TO THE RAIL COMPANY BY THE CONTRACTOR. THE COSTS FOR WATCHMEN OR FLAGGERS OCCASIONED BY THE NEGLIGENCE OF THE CONTRACTOR, OR ANY SUB-CONTRACTOR, IN CONNECTION WITH THE INSTALLATION OF THE PIPE OR CONDUIT SHALL BE PAID BY THE CONTRACTOR.

TRACK SUPPORTS REQUIRED BY THE RAIL COMPANY IN CONNECTION WITH THE INSTALLATION OF THE PIPE OR CONDUIT PER THE PLAN SHALL BE INCLUDED IN THE COMPANY FORCE ACCOUNT WORK AND PAID BY THE STATE. THE COST OF ANY TRACK SUPPORTS REQUIRED BY AN ALTERNATE METHOD OF INSTALLATION OF THE PIPE OR CONDUIT SHALL BE SHALL BE PAID TO THE RAIL COMPANY BY THE CONTRACTOR.

THE CONTRACTOR SHALL SECURE APPROVAL OF HIS OPERATIONS FROM THE STATE AND THE RAIL COMPANY. THE RAIL COMPANY WILL PERFORM AN ENGINEERING REVIEW OF METHODS OF OPERATIONS AND ENGINEERING SUPERVISION OF CONSTRUCTION WITHOUT COST TO THE CONTRACTOR.

PRIOR TO BIDDING, THE CONTRACTOR SHALL COORDINATE WITH THE RAIL COMPANY TO AGREE UPON THE REQUIREMENTS OF WATCHMEN AND FLAGGERS TO PROTECT RAILROAD TRAFFIC DURING THE CONTRACTOR'S OPERATIONS. THE CONTRACTOR SHALL EXECUTE A BOND IN FAVOR OF BOTH THE STATE AND THE COMPANY AS REQUIRED BY SECTION 5525.16 OF THE REVISED CODE OF OHIO.

THE CONTRACTOR SHALL CO-OPERATE WITH THE RAILROAD OFFICIALS CONCERNING WORK ADJACENT TO RAILROAD TRACKS, IN ORDER TO AVOID DELAY TO, OR INTERFERENCE WITH RAILROAD TRAFFIC, AND SHALL NOTIFY THE COMPANY 48 HOURS IN ADVANCE OF CONSTRUCTION OPERATIONS.

MONUMENT ASSEMBLIES

CONSTRUCT MONUMENT ASSEMBLIES IN ACCORDANCE WITH THE DETAILS SHOWN ON THE STANDARD CONSTRUCTION DRAWINGS AND AT THE LOCATIONS SHOWN ON SHEET RW.10.

EAR	THWORK QUANTITY	SUBSUMMARY										
	2	03	659									
ROAD	EXCAVATION	EMBANKMENT	SEEDING AND MULCHING									
	СҮ	СҮ	SY	_								
US 33 (01/NHS/01)	14589	10906	34391									
PICKERINGTON (04/STR/04)	21352	146323	59405	_								
RAMP A (01/NHS/01)	1587	17204	7798	_								
RAMP B (01/NHS/01)	4949	22619	13895	DESIGN AG	IENCY							
RAMP C (01/NHS/01)	1108	2513	4301									
RAMP D (01/NHS/01)	1875	9472	8644	CARPENTER.	sportati							
NORTH CONNECTOR (04/STR/04)	20745	8254	38699	EN	Y tran							
SOUTH CONNECTOR (04/STR/04)	19153	29204	53334	LRP	ARI							
ALLEN RD (01/NHS/01)	837	147	2762	CA	W							
ALLEN RD (01/NHS/01)	234	132	1281									
SERVICE RD 1 (04/STR/04)	2462	4194	7345		DW EWER							
SERVICE RD 2 (04/STR/04)	5212	2638	11085		2/09/24							
DRIVE 1 (04/STR/04)	672	3970	3226) 555							
KING DITCH (04/STR/04) 1382 27 3572												
CARRIED TO GENERAL SUMMARY	96157	257603	249738	P.25	total 846							

BENCHING OF FOUNDATION SLOPES

ALTHOUGH CROSS-SECTIONS INDICATE SPECIFIC DIMENSIONS FOR PROPOSED BENCHING OF THE EMBANKMENT FOUNDATIONS IN CERTAIN AREAS, NO WAIVER OF THE SPECIFICATIONS IS INTENDED. BENCH ALL OTHER SLOPED EMBANKMENT AREAS AS SET FORTH IN SECTION 203.05 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS (C&MS). NO ADDITIONAL PAYMENT WILL BE MADE FOR BENCHING REQUIRED UNDER THE **PROVISIONS OF SECTION 203.05.**

ITEM 614, WORK ZONE IMPACT ATTENUATOR FOR 24" WIDE HAZARDS (UNIDIRECTIONAL OR BIDIRECTIONAL)

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING A NON-GATING IMPACT ATTENUATOR. FURNISH AN IMPACT ATTENUATOR FROM THE OFFICE OF ROADWAY ENGINEERING'S APPROVED LIST FOR WORK ZONE IMPACT ATTENUATORS, FROM THE ROADWAY STANDARDS APPROVED PRODUCTS WEB PAGE.

INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

THE CONTRACTOR SHALL REPAIR OR REPLACE A DAMAGED UNIT WITHIN 24 HOURS OF A DAMAGING IMPACT.

WHEN BIDIRECTIONAL DESIGNS ARE SPECIFIED, THE CONTRACTOR SHALL SUPPLY APPROPRIATE TRANSITIONS.

WHEN GATING IMPACT ATTENUATORS ARE DESIRED, THE CONTRACTOR SHALL SUBMIT DOCUMENTATION TO THE ENGINEER FOR ACCEPTANCE.

THE COST FOR THE ADDITIONAL BARRIER REQUIRED FOR A GATING IMPACT ATTENUATOR SHALL BE INCLUDED IN THE COST OF THE GATING IMPACT ATTENUATOR.

PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT AND MAINTAIN A COMPLETE AND FUNCTIONAL IMPACT ATTENUATOR SYSTEM, INCLUDING ALL RELATED BACKUPS, TRANSITIONS, LEVELING PADS, HARDWARE AND GRADING, NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

WORK ZONE MARKINGS

THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN CARRIED TO THE GENERAL SUMMARY FOR USE AT LOCATIONS IDENTIFIED BY THE ENGINEER FOR WORK ZONE PAVEMENT MARKINGS PER THE REQUIREMENTS OF C&MS 614.11.

ITEM 614, WORK ZONE LANE LINE, CLASS III, 6", 642 PAINT	4.39 MILE
ITEM 614, WORK ZONE CENTER LINE, CLASS III, 642 PAINT	2.88 MILE
ITEM 614, WORK ZONE EDGE LINE, CLASS III, 4", 642 PAINT	4.92 MILE
ITEM 614, WORK ZONE EDGE LINE, CLASS III, 6", 642 PAINT	10.86 MILE
ITEM 614, WORK ZONE CHANNELIZING LINE, CLASS III, 8",	
642 PAINT	1311 FT
ITEM 614, WORK ZONE CHANNELIZING LINE, CLASS III, 12",	
642 PAINT	5381 FT
ITEM 614, WORK ZONE STOP LINE, CLASS III, 642 PAINT	389 MILE
ITEM 614, WORK ZONE ARROW, CLASS III, 642 PAINT	31 EACH

POWERED DEVICES

THESE SPECIFICATIONS APPLY TO THE SOLAR POWERED LED SIGNAL AHEAD WARNING SIGNS (W3-3-48) AT STA.176+20 FOR US-33 WESTBOUND.

RUN REQUIREMENTS OF THIS DEVICE ARE 24 HOURS PER DAY, 7 DAYS PER WEEK.

UTILIZE ENVIRONMENTALLY-SEALED, HIGH-EFFICIENCY LED LIGHT SOURCES FOR THIS SOLAR-POWERED APPLICATION.

HOUSE THE SOLAR POWER SUPPLY CONTROLLER AND BATTERY IN ONE OR TWO STAINLESS STEEL OR ALUMINUM ENCLOSURES WITH A MINIMUM NEMA 3 OR 3X RATING.

IF THE EXTERIOR SIZE OF THE ENCLOSURE NECESSARY TO MEET THE REQUIREMENTS BELOW IS LESS THAN 1000 CUBIC INCHES, A SINGLE POLYMER ENCLOSURE RATED NEMA 4 AND LISTED AS SUNLIGHT-RESISTANT MAY BE INSTALLED, WITH APPROVAL OF THE ENGINEER.

SEAL ENCLOSURE CONDUIT ENTRIES TO PREVENT INSECT AND/OR RODENT ENTRY. PROVIDE METAL ENCLOSURES WITH AN EXTERIOR OF BARE OR POWDER-COATED ALUMINUM, OR STAINLESS STEEL.

PROVIDE A LOCKING ENCLOSURE USING EITHER AN INTEGRATED LOCKING MECHANISM OR A PADLOCK PER C&MS 631.06.

SMALL ENCLOSURES OF 300 CUBIC INCHES OR LESS (EXTERIOR) MAY BE PROVIDED WITH SECURITY FASTENERS IN LIEU OF A LOCKING MECHANISM OR PADLOCK.

SEPARATE THE CONTROL ELECTRONICS AND BATTERY, IF CONTAINED WITHIN A SINGLE ENCLOSURE, TO PREVENT DAMAGE TO THE CONTROL ELECTRONICS IF THE BATTERY ENVELOPE IS COMPROMISED.

PROVIDE SEALED GEL-CELL OR AGM (ABSORBED GLASS MAT) LEAD-ACID BATTERIES FOR ALL INSTALLATIONS WITH INSTANTANEOUS LOAD REQUIREMENTS OF 4 WATTS OR ABOVE, REGARDLESS OF DUTY CYCLE.

FOR INSTALLATIONS WITH INSTANTANEOUS LOAD REQUIREMENTS OF LESS THAN 4 WATTS, RECHARGEABLE NICD, LI-ION, OR NIMH BATTERIES MAY BE USED INSTEAD OF AGM OR GEL-CELL, IF APPROVED BY THE ENGINEER.

PROVIDE SIGNED COPIES FROM THE SOLAR PANEL AND/OR CONTROLLER MANUFACTURER OF ALL CALCULATIONS USED TO SIZE THE SOLAR PANEL AND BATTERIES.

INCLUDE IN THESE CALCULATIONS THE INSOLATION VALUE USED AND ITS REFERENCE SOURCE, THE SOLAR PANEL EFFICIENCY, CHARGER/CONTROLLER EFFICIENCY, INVERTER EFFICIENCY, PROPOSED LED LAMP AND/OR EQUIPMENT LOAD, AND A FIGURE REPRESENTING ANTICIPATED MISCELLANEOUS LOSSES.

SHOW CALCULATIONS DOCUMENTING A RESERVE CAPACITY OF TWO WEEKS OPERATION UNDER CONTINUOUS WORST-CASE (MINIMUM) INSOLATION FIGURES (USUALLY DECEMBER) FOR THE PROPOSED GEOGRAPHIC LOCATION, USING A PANEL ELEVATION ANGLE APPROPRIATE TO THE SITE, AT A SUSTAINED TEMPERATURE OF 25 DEGREES FAHRENHEIT (-4 DEGREES CELSIUS).

DELIVER A COPY OF THE CALCULATIONS TO THE ENGINEER AND ANOTHER COPY TO THE OFFICE OF ROADWAY ENGINEERING FOR APPROVAL.

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GENERAL ELECTRICAL REQUIREMENTS FOR SOLAR-

PROVIDE DOCUMENTATION SHOWING THAT THE SOLAR PANEL MANUFACTURER TESTED THE PANEL ACCORDING TO IEC61215 OR EQUIVALENT APPROVED STANDARD.

PROVIDE DOCUMENTATION SHOWING THAT SOLAR PANEL MOUNTING IS RATED FOR 90 MPH DESIGN WIND AND DESIGNED TO RESIST VANDALISM.

ENSURE NEC GROUNDING AND BONDING REQUIREMENTS ARE MET IF VOLTAGES OVER 50V AC OR DC ARE PRESENT.

PROVIDE A TIMER (IF REQUIRED) THAT SATISFIES THE REQUIREMENTS OF C&MS 731.10 AND IS LISTED ON THE ODOT QUALIFIED PRODUCTS LIST.

PROVIDE COMPLETE PHOTO-CONTROLLER SPECIFICATIONS, INCLUDING ON/OFF PHOTOMETRIC SWITCH POINTS (TYPICALLY GIVEN IN FOOT-CANDLES), IF A PHOTO-CONTROLLER IS UTILIZED.

PAYMENT FOR INSTALLING, MAINTAINING, AND REMOVING THESE SIGNS, INCLUDING ALL LABOR, EQUIPEMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM, 614 MAINTAINING TRAFFIC.

MAINTENANCE OF TRAFFIC GENERAL NOTES
ARTY transportation
DESIGNER CTF REVIEWER NAU 12/09/24 PROJECT ID 77555 SHEET TOTAL P.32 846

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P.23	P.24	P.25	P.139B	P.139G	P.146	P.147	P.148	P.149	P.150	P.453	RW.10		01/NHS/01	02/NHS/08 03/NHS/13	04/STR/04		EXT	TOTAL	ONIT		NO.	
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			15,253	7,666	1,057	2,954			_				16,310		10,620	202	23000	26,930	SY	PAVEMENT REMOVED		_
					4,020	1,226									5,246	202	30000	5,246	SF	WALK REMOVED		
					212	743			1,375				693		1,637	202	35100	2,330	FT	PIPE REMOVED, 24" DIAMETER AND UNDER		
						26			66				66		26	202	35200	92	FT	PIPE REMOVED, OVER 24" DIAMETER		
					956	521			00				956		521	202	38000	1,477	FT	GUARDRAIL REMOVED		
					14	14							14		14	202	53100	28	EACH	MAILBOX REMOVED		
						3									3	202	58000	3	EACH	MANHOLE REMOVED		_
						1			6				4		3	202	58100	/	EACH	CATCH BASIN REMOVED		_
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			4,476	25,412									4,476		25,412	206	15010	29,888	SY	CEMENT STABILIZED SUBGRADE, 12 INCHES DEEP		
			21,368	2,684									21,368		2,684	206	15020	24,052	SY	CEMENT STABILIZED SUBGRADE, 14 INCHES DEEP		
													LS		LS	206	30000	LS		MIXTURE DESIGN FOR CHEMICALLY STABILIZED SOILS		
							4,898	2,450					4,898		2,450	606	15050	7,348	FT	GUARDRAIL, TYPE MGS		
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							1						1			606	60002	1		IMPACT ATTENUATOR, TYPE 1 (UNIDIRECTIONAL)		
							621						 621			606 607	60028 15000	621	EACH FT	IMPACT ATTENUATOR, TYPE 2 (BIDIRECTIONAL) 70 MPH, 24" WIDE FENCE, TYPE 47		_
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							4,206	1,312					4,206		1,312	607	70000	5,518	FT	FENCELINE SEEDING AND MULCHING		DESI
							156 2,040						156 2,040			622 622	10100 10160	156 2,040	FT FT	CONCRETE BARRIER, SINGLE SLOPE, TYPE B1 CONCRETE BARRIER, SINGLE SLOPE, TYPE D		
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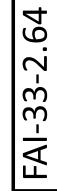
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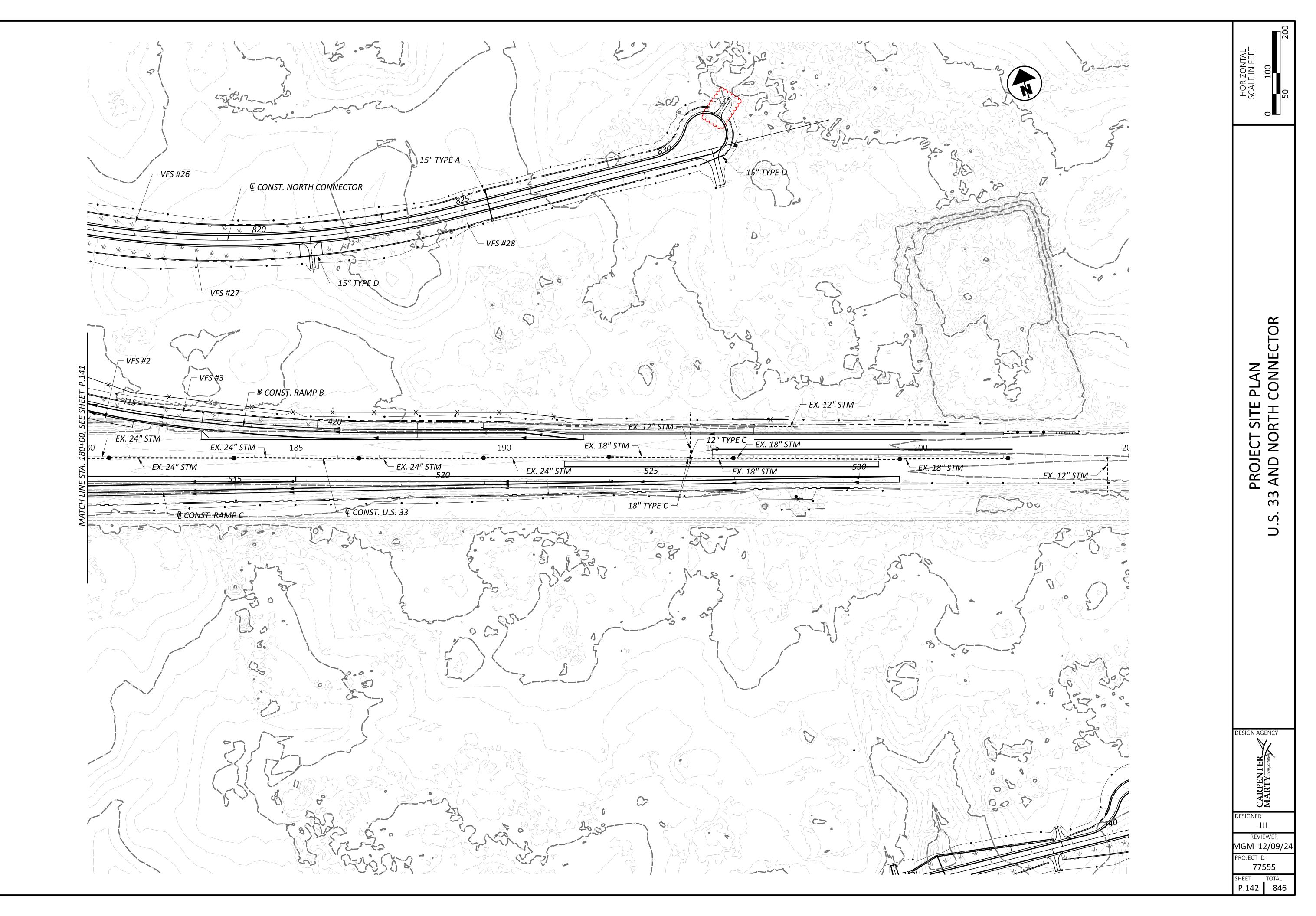
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2 SPECIAL 63820884 2 EACH CUT AND PLUG EXISTING 12" WATER	_				1	638	11102	1	EACH	METER AND VAULT REMOVED AND RE
					1,035	SPECIAL	63820768	1,035	FT	³ / ₄ " POLYETHYLENE WATER SERVICE I
13 SPECIAL 63820904 13 EACH SERVICE BOX (FAIRFIELD COUNTY)										CUT AND PLUG EXISTING 12" WATER
					13	SPECIAL	63820904	13	EACH	SERVICE BOX (FAIRFIELD COUNTY)

DESCRIPTION	SEE SHEET NO.	
DRAINAGE CONTINUED		
IAGE CONTINUANCE	P.24	
NAGE CONTINUANCE	P.24	
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IAGE CONTINUANCE	P.24	
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		GENERAL SUMMARY
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OUTLET		AX
001221		
		Z
PAVEMENT		
CRETE (DEPTH = 3 1/4")		
2, (449)		
2, (449), (DRIVEWAYS)		
JRSE, TYPE 1, (446), PG70-22M		
E COURSE, TYPE 2, (446)		
JRSE, TYPE 1, (449), (DRIVEWAYS)		
E COURSE, TYPE 2, (449), (DRIVEWAYS)		
JRSE, 12.5 MM, TYPE A (446)		
E COURSE, 12.5 MM, TYPE A (446)		
ENT, CLASS QC 1P		
AVEMENT, CLASS QC 1P		
VEMENT, CLASS QC 1P WITH QC/QA		
WATER WORK		
DE PIPE AND FITTINGS, AWWA C900, DR14		
DE PIPE AND FITTINGS, AWWA C900, DR14		
IDE PIPE AND FITTINGS, AWWA C900, DR-14		DESIGN AGENCY
ED OR JACKED		\vee
		Hatton Hatton
		TTE
		PEN
ND VALVE BOX		CARI MAR
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POSED OF		DESIGNER
		MGM
RESET		REVIEWER TWG 12/17/24
LINE (FAIRFIELD COUNTY)	P.551	PROJECT ID
R LINE (FAIRFIELD COUNTY)	P.547	77555
	P.551	SHEET TOTAL
		P.130 846

		SHEET	NUM.		<u>{</u>	3			PA	RT.			ITEM	GRAND		
P.25	P.27	P.29	P.30	P.31	P.32	P.42	P.47	01/NHS/01	02/NHS/08	03/NHS/13	04/STR/04	ITEM	EXT	TOTAL	UNIT	DI
					Uuuuu		5.05				5.0.5	202	25400	505		
	1,922						585	1,922			585	202 254	35100 01000	585 1,922	FT SY	PIPE REMOVED, 24" DIAMETER AND UNDER PAVEMENT PLANING, ASPHALT CONCRETE (DEPTH =
	1,922		10					1,922			10	407	20000	1,922	GAL	NON-TRACKING TACK COAT
	100							100			100	410	12000	100	CY	TRAFFIC COMPACTED SURFACE, TYPE A OR B
	100										100	410	13000	100	СҮ	TRAFFIC COMPACTED SURFACE, TYPE C
			50								50		70000	50		
	80		50					80			50	441 442	70000 20000	50 80	CY CY	ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (449) ASPHALT CONCRETE SURFACE COURSE, 12.5 MM, TY
	80						154	00			154	611	04200	154	FT	12" CONDUIT, TYPE A, 706.02
							318				318	611	04900	318	FT	12" CONDUIT, TYPE D
							35				35	611	05900	35	FT	15" CONDUIT, TYPE B
							78				78	611	07200	78	FT	18" CONDUIT, TYPE A
		1.000						000			200	C1 A	11110	1 000		
		1,000		1				800			200	614 SPECIAL	11110 61411300	1,000	HOUR EACH	LAW ENFORCEMENT OFFICER WITH PATROL CAR FOF WORK ZONE TRAFFIC SIGNAL
						18		18				614	12380	18	EACH	WORK ZONE IMPACT ATTENUATOR, 24" WIDE HAZAF
						17		10			17	614	12380	10	EACH	WORK ZONE IMPACT ATTENUATOR, 24" WIDE HAZAF
	LS										LS	614	12420	LS		DETOUR SIGNING
	5							5				614	12500	5	EACH	REPLACEMENT SIGN
	25							25				614	12600	25	EACH	REPLACEMENT DRUM
 			50			2,122		2,122				614	12800	2,122	EACH	WORK ZONE RAISED PAVEMENT MARKER
	50		50			387		10 387			90	614 614	13000 13310	100 387	CY EACH	ASPHALT CONCRETE FOR MAINTAINING TRAFFIC BARRIER REFLECTOR, TYPE 1, ONE WAY
						507		50/				014	01551	507		DANNEN NEI LLOION, TIFE 1, UNE WAT
						76					76	614	13310	76	EACH	BARRIER REFLECTOR, TYPE 1, BIDIRECTIONAL
						387		387				614	13350	387	EACH	OBJECT MARKER, ONE WAY
						76					76	614	13360	76	EACH	OBJECT MARKER, TWO WAY
		36						100,000				614 614	18000 18601	100,000	EACH	MAINTAINING TRAFFIC, MISC.: SAFETY REPAIRS
		30						36				014	18601	36	SNMT	PORTABLE CHANGEABLE MESSAGE SIGN, AS PER PLA
						2.38		2.38				614	20056	2.38	MILE	WORK ZONE LANE LINE, CLASS I, 6", 807 PAINT
					<i>4.39</i>			4.39		*****		614	20560	4.39	MILE	WORK ZONE LANE LINE, CLASS III, 6", 642 PAINT
						4.3			~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		4.3	614	21100	4.3	MILE	WORK ZONE CENTER LINE, CLASS I, 642 PAINT
					{ 2.88	<i>9.75</i>		0.04 9.75			2.84	614 614	21550 22056	2.88 9.75	MILE	WORK ZONE CENTER LINE, CLASS III, 642 PAINT
 						3.13		3.75				014	22030	5.13	IVIILE	WORK ZOINL LUGE LINE, CLASS I, D , 807 PAINT
						8.18					8.18	614	22110	8.18	MILE	WORK ZONE EDGE LINE, CLASS I, 6", 642 PAINT
					<i>4.92</i>						4.92	614	22350	4.92	MILE	WORK ZONE EDGE LINE, CLASS III, 4", 642 PAINT
					ξ 10.86			10.86				614	22360	10.86	\dots	WORK ZONE EDGE LINE, CLASS III, 6", 642 PAINT
 						17,781		17,781			0.25	614	23110	17,781	FT	WORK ZONE CHANNELIZING LINE, CLASS I, 12", 807 I
					<i>E</i> 1,311	925					925 1,311	614 614	23200 23680	925 1,311	FT FT	WORK ZONE CHANNELIZING LINE, CLASS I, 8", 642 PA WORK ZONE CHANNELIZING LINE, CLASS III, 8", 642 PA
					5,381			5,381			1,511	614	23690	5,381	FT FT	WORK ZONE CHANNELIZING LINE, CLASS III, 8 , 642 F
															······	
						1,936		1,936				614	24102	1,936	FT	WORK ZONE DOTTED LINE, CLASS I, 6", 807 PAINT
						3,471					3,471	614	25200	3,471	FT	WORK ZONE TRANSVERSE/DIAGONAL LINE, CLASS I,
 					[454		88	······		366	614	26200	454	FT	WORK ZONE STOP LINE, CLASS I, 642 PAINT
 					<u> </u>	36		78			311	614 614	26610 30200	389 36	FT EACH	WORK ZONE STOP LINE, CLASS III, 642 PAINT
 					<i>§ 31</i>	50		26 15	~~~~~~	~~~~~~	10 16	614 614	30200	36 31	EACH	WORK ZONE ARROW, CLASS 1, 642 PAIN 1 WORK ZONE ARROW, CLASS III, 642 PAINT
						2					2	614	32200	2	EACH	WORK ZONE RAILROAD SYMBOL MARKING, CLASS I,
												.				
						12 502					LS 12.502	615	10000	LS 12502		ROADS FOR MAINTAINING TRAFFIC
 	1,100					12,502		500			12,502 600	615 616	20000 10000	12,502 1,100	SY MGAL	PAVEMENT FOR MAINTAINING TRAFFIC, CLASS A WATER
 	1,100		50								50	616	10000	1,100 50	CY MGAL	COMPACTED AGGREGATE
-			10	1							10	617	25000	10	MGAL	WATER
	8,650							8,650				618	40100	8,650	FT	RUMBLE STRIPS, SHOULDER (ASPHALT CONCRETE)
			1			21,810		18,590			3,220 1	622 642	41100	21,810	FT MILE	PORTABLE BARRIER, UNANCHORED
				144				144			1	642 808	00300 18700	1 144	MILE SNMT	CENTER LINE, TYPE 1 DIGITAL SPEED LIMIT (DSL) SIGN ASSEMBLY
													10,00	<u></u> ∡ ⊤ 7		
 · · · · · · · · · · · · · · · · · · ·								LS				108	10000	LS		CPM PROGRESS SCHEDULE
			_			1	I	LS				614	11000	LS	I	MAINTAINING TRAFFIC
	LS										I				A A 1	
	LS							36				619	16020	36	MNTH	FIELD OFFICE, TYPE C
LS	LS														MNTH	

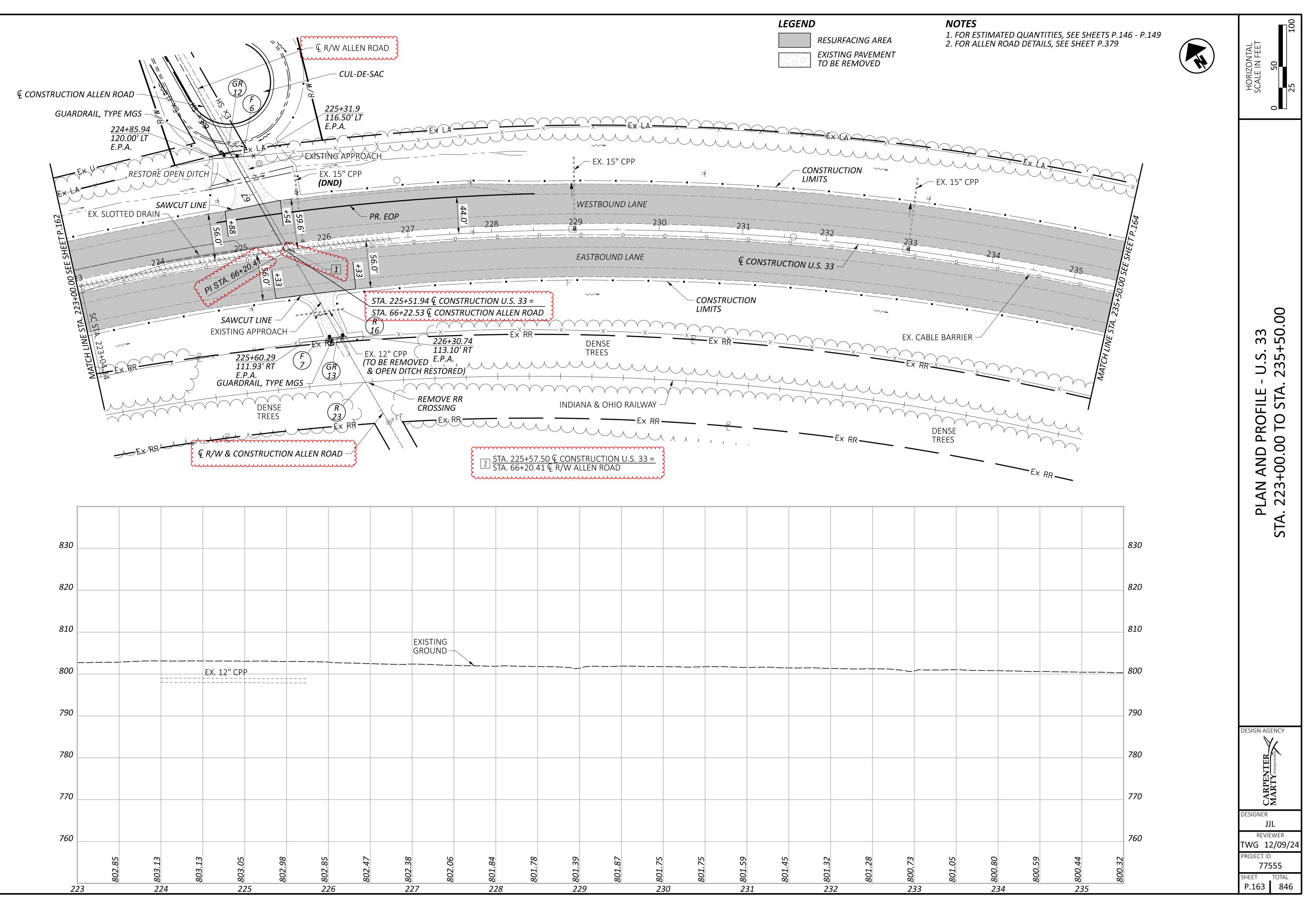
DESCRIPTION	SEE SHEET NO.	
ITENANCE OF TRAFFIC		
1 = 1.5")		
7 – 1.3)		
49), PG64-22		
TYPE A (448)		
FOR ASSISTANCE	P.31	
ZARDS, (UNIDIRECTIONAL)	F.J1	
ZARDS, (BIDIRECTIONAL)		
		GENERAL SUMMARY
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PLAN	P.31 P.29	U Z
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51, 642 PAINT		
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51, 642 PAINT		
		DESIGN AGENCY
)		NTER (transportation
		RPE
		CA MA
		designer MGM
INCIDENTALS		REVIEWER TWG 12/17/24
		PROJECT ID
, AS PER PLAN	P.25	77555 SHEET TOTAL
		P.135 846

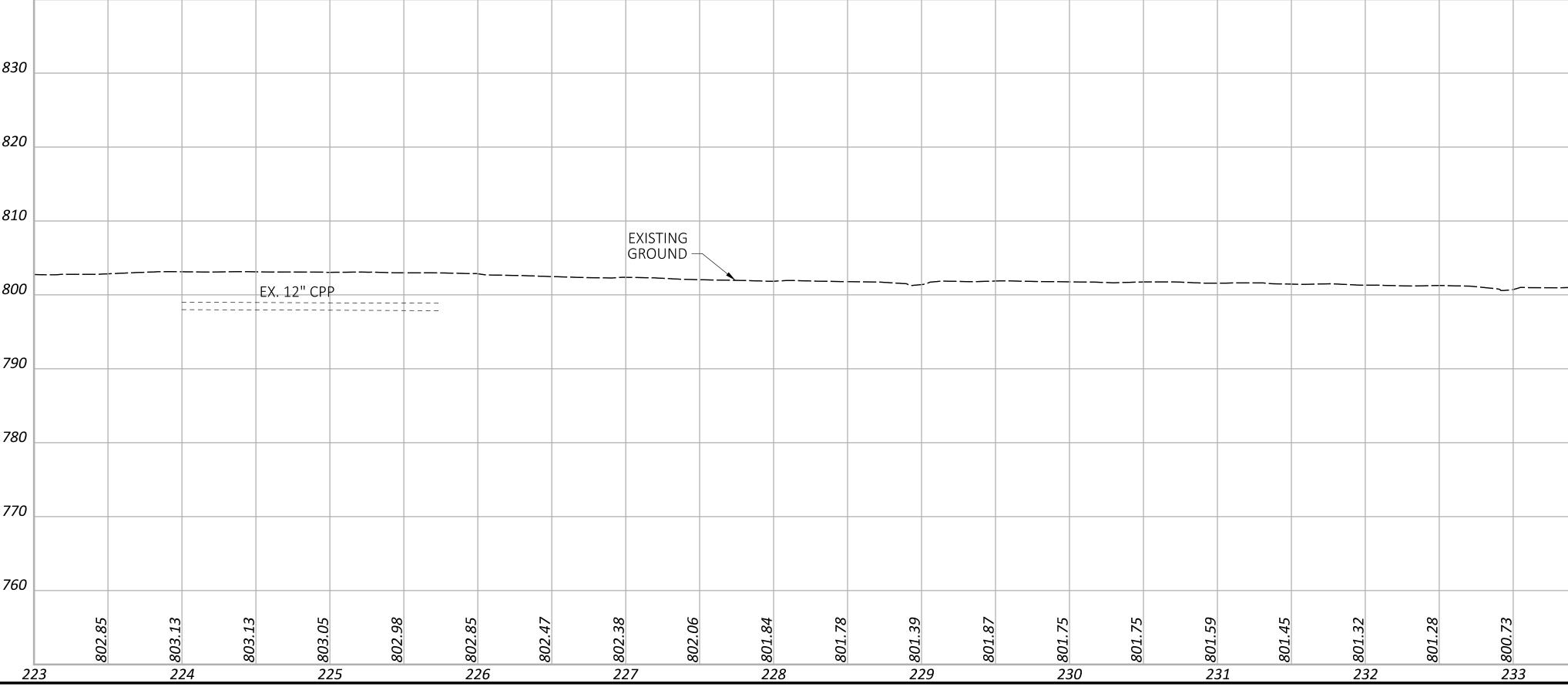




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RE		SHEET NO.	LOCATION	STA	TION	SIDE	RIPRAP, TYPE D	ROCK CHANNEL PROTECTIO TYPE A WITH FILTER	ROCK CHANNEL PROTECTIO TYPE B WITH FILTER	ROCK CHANNEL PROTECTIO TYPE C WITH FILTER	CONCRETE MASONRY	12" CONDUIT, TYPE C	15" CONDUIT, TYPE B	15" CONDUIT, TYPE C	18" CONDUIT, TYPE C	21" CONDUIT, TYPE B	21" CONDUIT, TYPE C	24" CONDUIT, TYPE C	21"CONDUIT, TYPE A, 706.02 707.33 OR 24" CONDUIT, TYPE 707.01 (ALUMINIZED)	63" X 98" CONDUIT, TYPE A, 706.04	66" CONDUIT, TYPE B	6' X 4' CONDUIT, TYPE A, 706.05	CATCH BASIN, NO. 2-2B	CATCH BASIN, NO. 2-3	CATCH BASIN. NO. 3A	CATCH BASIN, NO. 6	CATCH BASIN, NO. 8	INLET, NO. 3D	SLOTTED DRAIN, TYPE 2, 15	MANHOLE, NO. 3	DITCH EROSION PROTECTIO	
				FROM	ТО	_	SY	СҮ	СҮ	СҮ	СҮ	FT	FT	FT	FT	EACH	EACH	EACH	EACH	EACH	EACH	FT	EACH	SY								
		D 474			104-00 74										40																405	
 		P.471 P.471	U.S. 33 U.S. 33	164+77.71 164+90.71	164+90.71 166+74.86	MED MED									13 184												1				125 125	
D-		P.472	U.S. 33	170+00.00	173+00.00	MED								298	101												1				125	
D-		P.473	U.S. 33	194+41.00	194+53.00	MED						6			12															1		
D-	-5	P.470	U.S. 33	148+80.38	151+97.00	RT								145												1			171			
D-	6	P.470	U.S. 33	151+97.00	153+52.47	RT							165										1									Ш Z
D- D-		P.470	U.S. 33	155+68.14	155+53.26	RT							105				17													1		
 D-		P.470	U.S. 33	156+49.20	155+68.14	RT											80							1								Z I
D-	-9	P.472	U.S. 33	173+00.00	174+63.23	RT							163																	1		A
D-1	10	P.472	U.S. 33	174+63.23	175+00.00	RT															38									1		Σ
D-*	11	D 170	110.00	175+00.00	175±00 00	στ		18			<u> </u>											60								<u> </u>		
D-1		P.472 P.472	U.S. 33 U.S. 33	175+00.00 176+08.75	175+09.83 176+08.75	RT RT		10			2.8			.9								60	1							£13		RY
D-1		P.472	RAMP A	308+50.00	308+94.59	LT/RT				2	0.9								112				,									<u>4</u>
D-1	14	P.273	RAMP B	404+89.09	405+68.10	LT/RT	16		19		4.8									142												
D-1	15	P.472	RAMP C	507+55.00	507+55.00	LT/RT							25												1							
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D-1 D-1		P.472 P.472	RAMP C RAMP C	507+55.00 507+55.00	507+55.00 507+55.00	RT RT				2	0.3			0 13											1					1		E E E E E E E E E E E E E E E E E E E
 D-1		P.470	RAMP D	607+74.22	608+58.17	RT			6		0.3			84									1							,		SU
D-1	19	P.470	RAMP D	608+96.77	609+80.00	RT		10			0.5						83							1								Щ
D-2	20	P.470	RAMP D	609+75.00	609+80.00	LT/RT										44												1				ÐØ
D-5	58	P.474	U.S. 33	195+39.26	195+59.13	LT/RT								0	Q												1					Ż
D-5		P.471	U.S. 33	158+71.00	159+96.00	LINKI				2	0.5			9	9			125									,					A A
D-6		P.471	U.S. 33	159+96.00	160+71.00	LT					0.5							75												1		DR.
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) 0-En <u>i</u>																																STY STY
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E: 34) 8/775																																
33-3.1																																BAC
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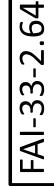
FAI-33-2.64 MODEL: Sheet PAPERSIZE: 34x22 (in) DATE: 5/5/2025 TIME: 3-25-49 DM



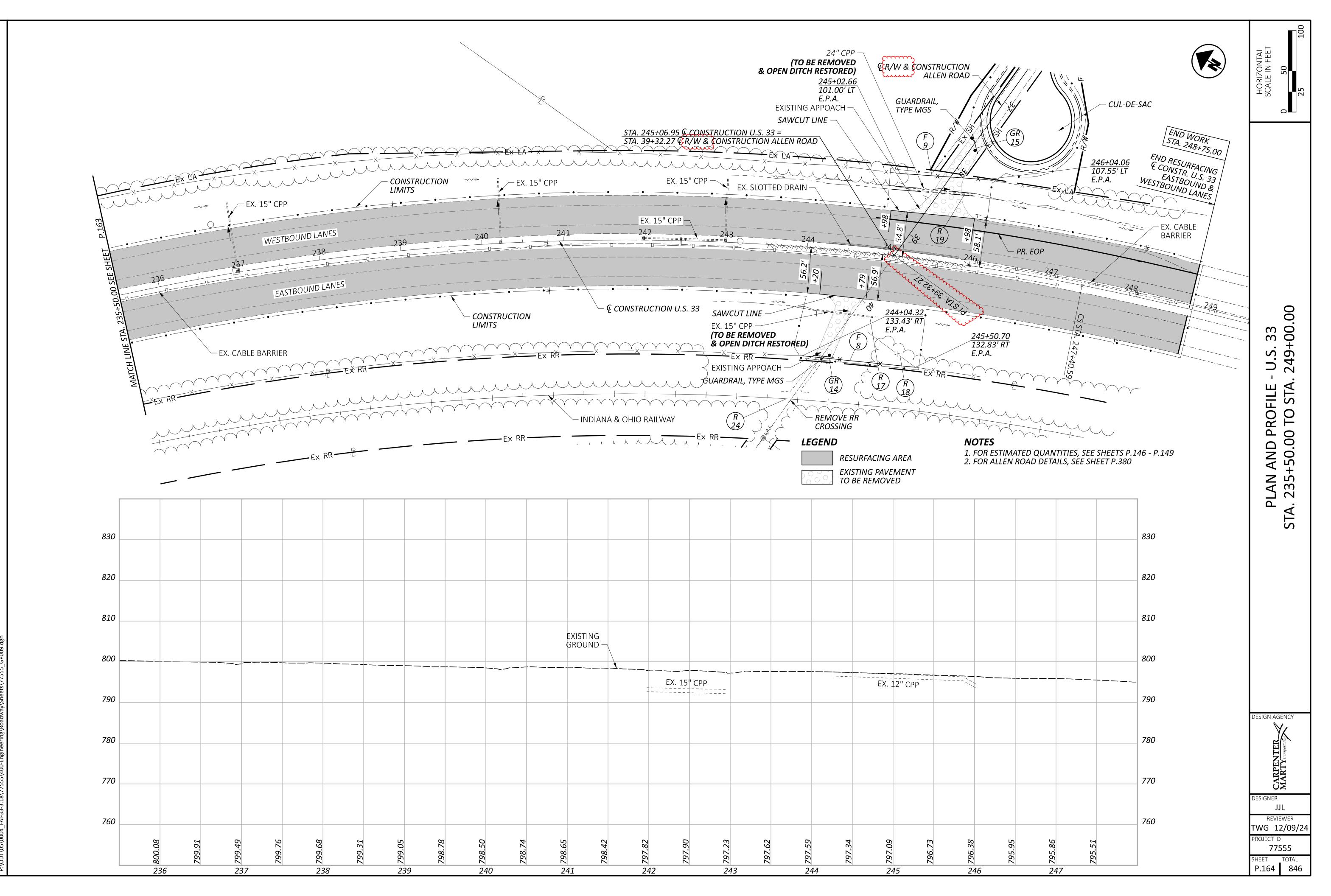


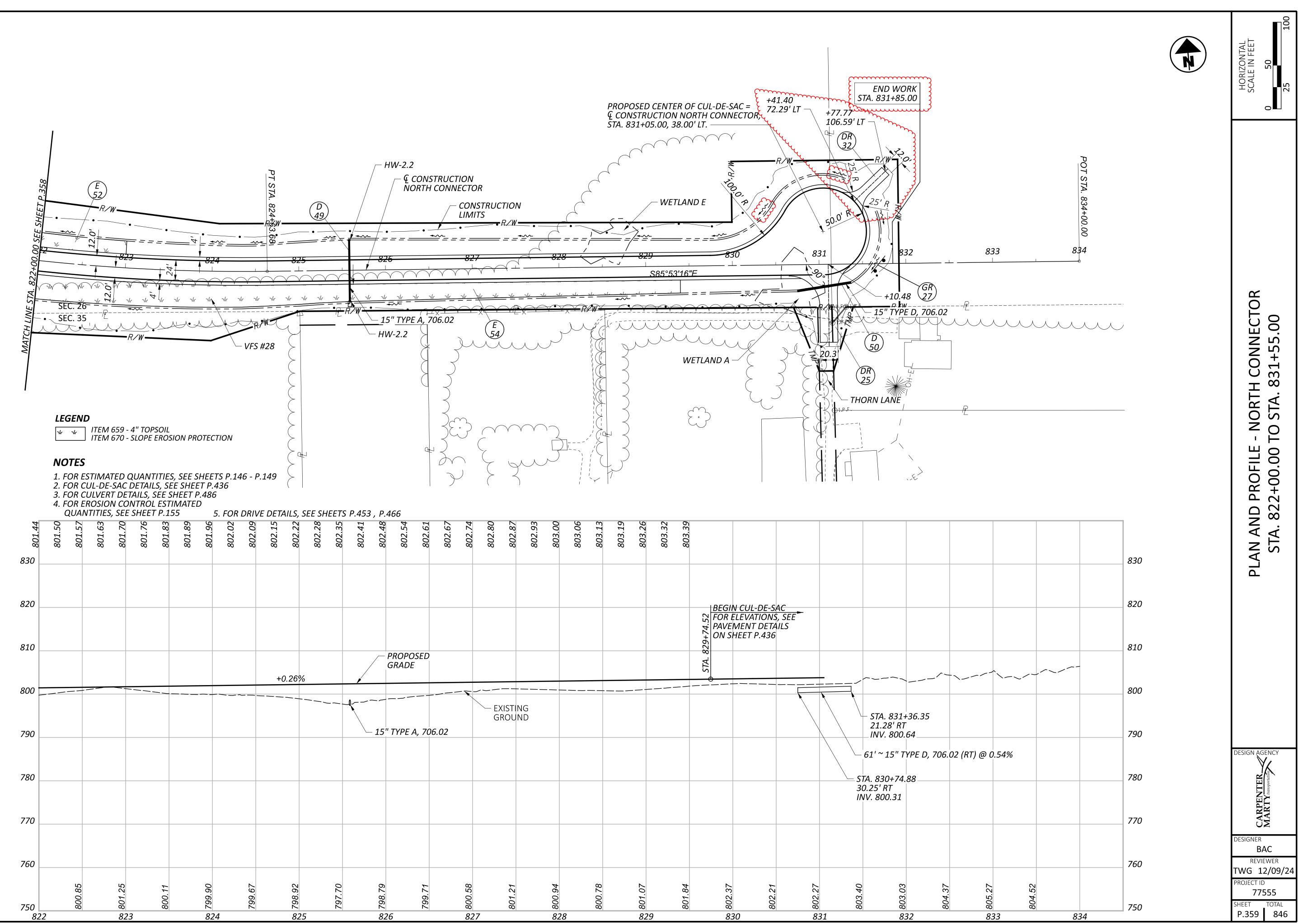
12 USE AΜ 49 1:41: TIME: 5/5/2025 DATE: 34x22 .64 ∞ \sim Ú. A





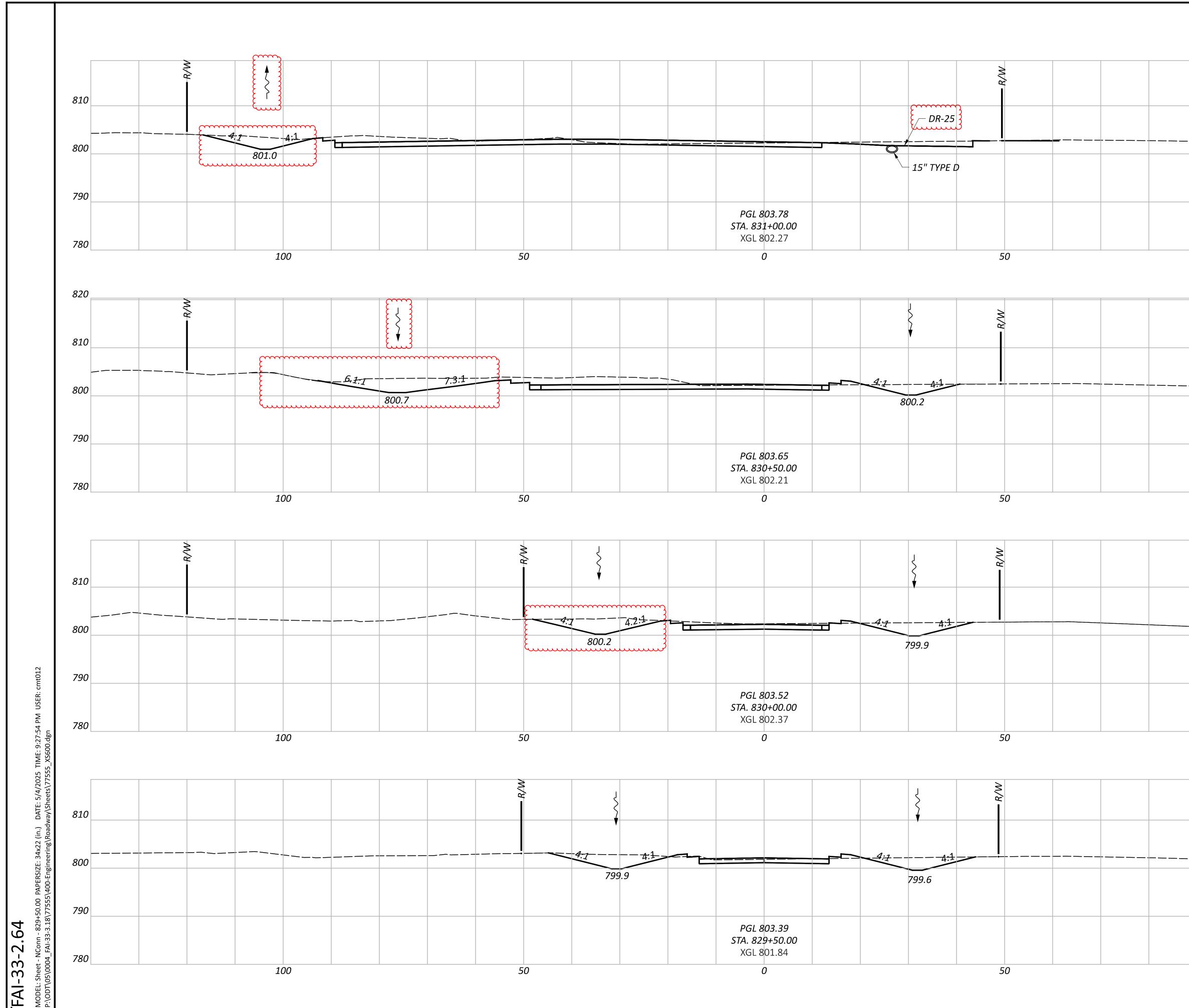
AODEL: CLX_U033 - Plan 9 PAPERSIZE: 34x22 (in.) DATE: 5/5/2025 TIME: 1:42:22 AM USER: cmt012 \\ODT\05\0004 FAI-33-3 18\77555\400-Fnøineerinø\Roadwav\Sheets\77555 GP009 døn





012 23 0 10 TIME: 9603.d DATE: 5/4/2025 ⁻ Sheets\77555_GP : 34x22 (in.) ng\Roadwav\ RSIZE: Plan

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01.10	802.54	802.61	802.67	802.74	802.80	802.87	802.93	803.00	803.06	803.13	803.19	803.26	803.32	803.39						
															BEGI FOR PAVE ON S	N CUL-DE-S ELEVATION MENT DET HEET P.436	SAC S, SEE AILS			
	PROPOS GRADE	SED													STA. 82				~~~~	
	TYPE A	, 706	5.02	-		ISTI KOU						· +							– STA. 831+ 21.28' RT INV. 800.	-
																			61' ~ 15" T TA. 830+74.	YPE D, 7
																		30	0.25' RT IV. 800.31	
0		799.71		800.58		801.21		800.94		800.78		801.07		801.84	802.37	802 21	802 27	803.40	803. <i>0</i> 3	
2	6	1		82		-		82		- 1		82	9	- 1		30		31		32



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		Cut Area (SF): { 58 }	Cut Vol. (C	Y): {182 {	
		Fill Area (S	SF): 26	Fill Vol. (C	Y): 31	
		Seed Widt	th (FT): 56	Seed Area	(SY): 3 58	
			TMP			810
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						800
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	Cut Area (SF): {138 }	Cut Vol. (C	Y): <mark>{</mark> 216	
	Fill Area (S	SF): 8	Cut Vol. (C Fill Vol. (C	Y): 9	
	Seed Widt	:h (FT): 73	Seed Area		
					810
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		Cut Area (SF): <mark>295 3</mark>	Cut Vol. (C	:Y): {157 }	
		Fill Area (S	SF): 2	Fill Vol. (C	Y): 11	
		Seed Widt	h (FT): 64	Seed Area	(SY): 3 64	
						810
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11	0				11	50
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		Cut Area ('SF):	74	Cut Vol. (C	CY): 10)2	
		Fill Area (S	SF):	10	Fill Vol. (C	Y): 40)	
		Seed Widt	:h (F1	Г): 67	Seed Area	(SY): 3 7	75 8	10
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1						She	et Tot	
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						1478	657	9 91

CROSS SECTIONS - NORTH CONNECTOR STA. 829+50.00 TO STA. 831+00.00 DESIGN AGENCY \bigvee CARPENTER MARTY transportation DESIGNER JJL REVIEWER

MGM 12/09/24

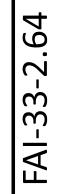
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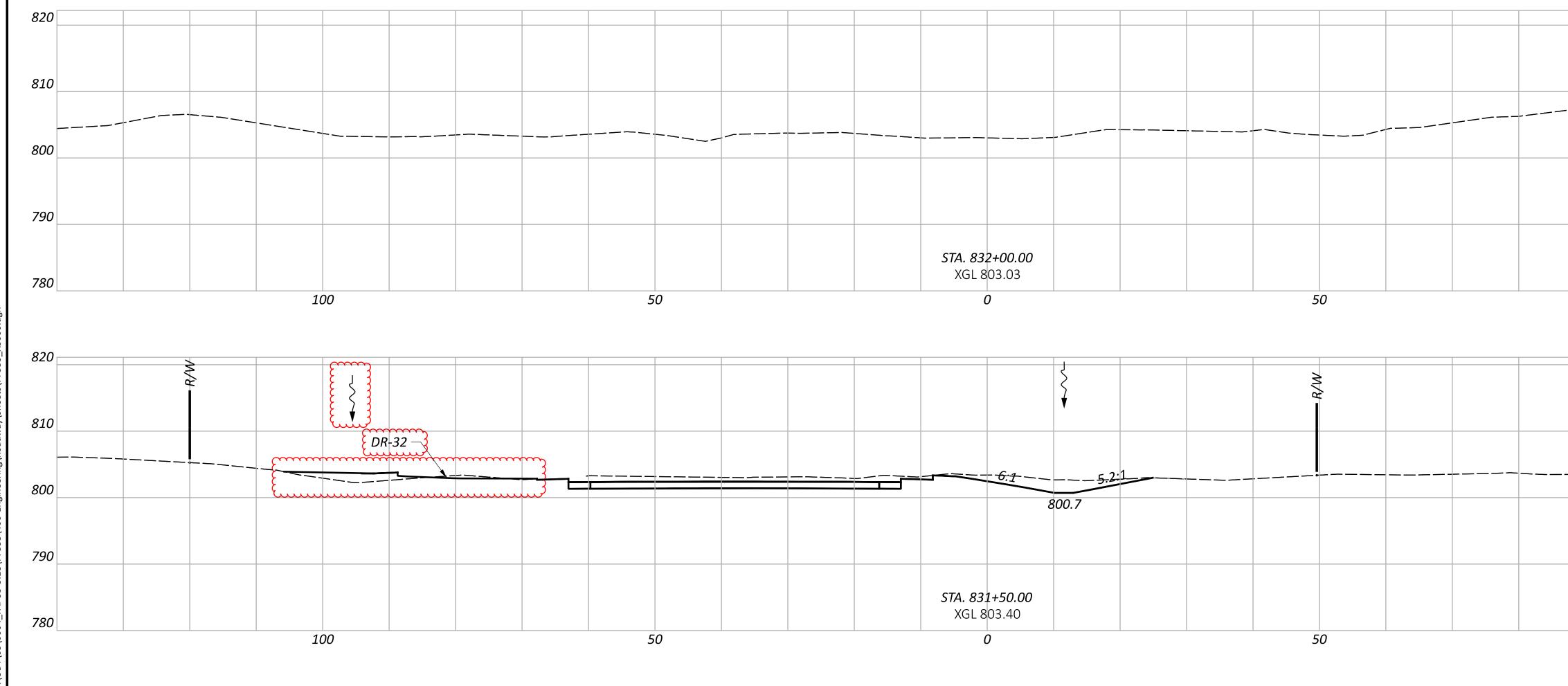
TOTAL

PROJECT ID

SHEET



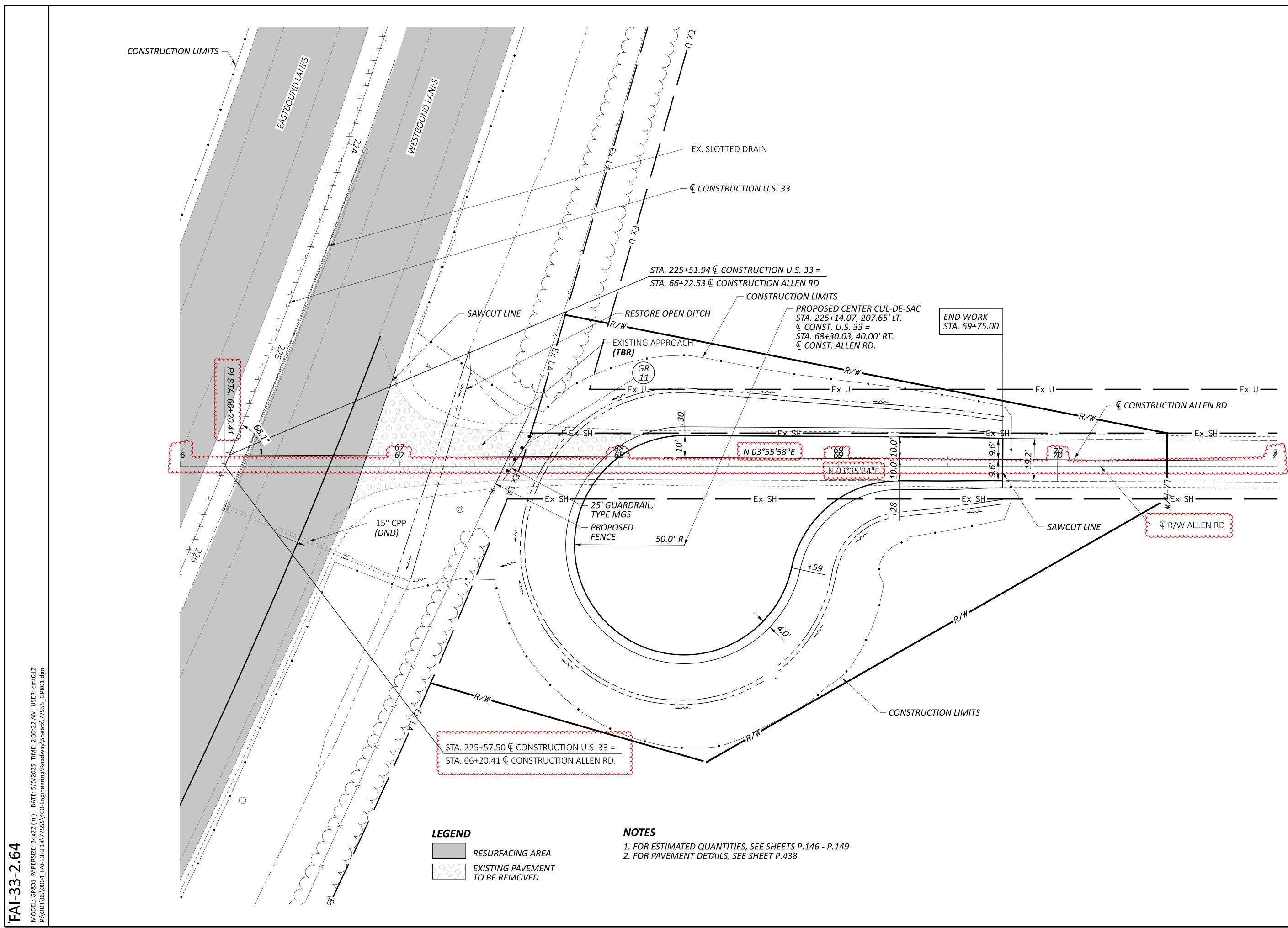
012 - 831+50.00 PAPERSIZE: 34x22 (in.) DATE: 5/4/2025 TIME: 9:29:56 PM USER:
 3-3.18\77555\400-Engineering\Roadway\Sheets\77555_X5600.dgn · NCoi 4 FAI ь. С Ē



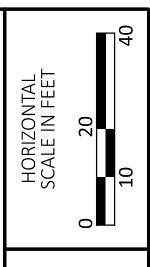


E	ARTHWORK QUANTI	TY SUBSUMMARY	
	2	03	659
SHEET	EXCAVATION	EMBANKMENT	SEEDING AND MULCHING
	СҮ	СҮ	SY
P.360	364	58	961
P.361	1087	21	1605
P.362	1590	13	2111
P.363	1938	1099	3422
P.364	2981	238	2934
P.365	2692	46	2000
P.366	2030	361	2689
P.367	1478	467	2539
P.368	1147	661	2333
P.369	812	805	2273
P.370	890	635	2222
P.371	344	910	1949
P.372	241	873	1812
P.373	1109	322	2539
P.374	720	361	2193
P.375	131	950	1533
P.376	332	293	1517
P.377	657	91	1478
P.378	202	£ 50 }	589
ARRIED TO SHEET P.25	20745	8254	38699

EA	<u>ΡΤΗ\Λ/ΛΟΥ ΛΙΙΛ ΑΙΤΙΤ</u>		
	RTHWORK QUANTITY 20		659
SHEET	EXCAVATION	EMBANKMENT	SEEDING AND MULCHING
	СҮ	СҮ	SY
P.360	364	58	961
P.361	1087	21	1605
P.362	1590	13	2111
P.363	1938	1099	3422
P.364 P.365	2981 2692	238 46	2934 2000
P.365 P.366	2032	361	2689
P.367	1478	467	2539
P.368	1147	661	2333
P.369	812	805	2273
P.370	890	635	2222
P.371	344	910	1949
P.372	241	873	1812
P.373	1109	322	2539
P.374	720	361	2193
P.375	131	950	1533
P.376	332	293	1517
P.377	657	91	1478
P.378 CARRIED TO SHEET P.25	202	8254	589 38699
	STA. 832+00.00 XGL 803.03		50
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		♦	F
	<u> </u>	<u>5.2:1</u>	
		800.7	
	STA. 831+50.00		
	<i>STA. 831+50.00</i> XGL 803.40 <i>0</i>		50







71+00.00

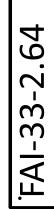
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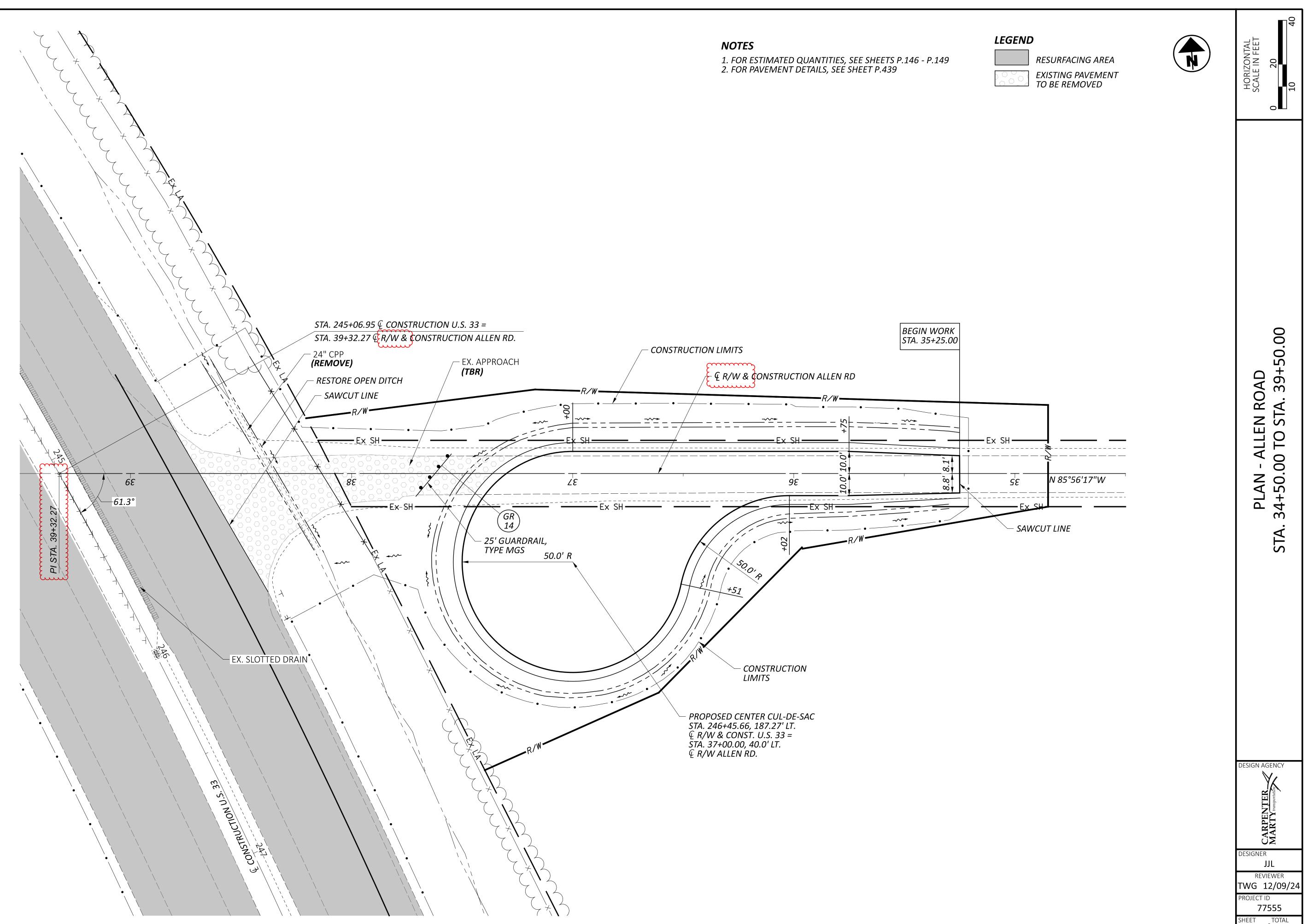
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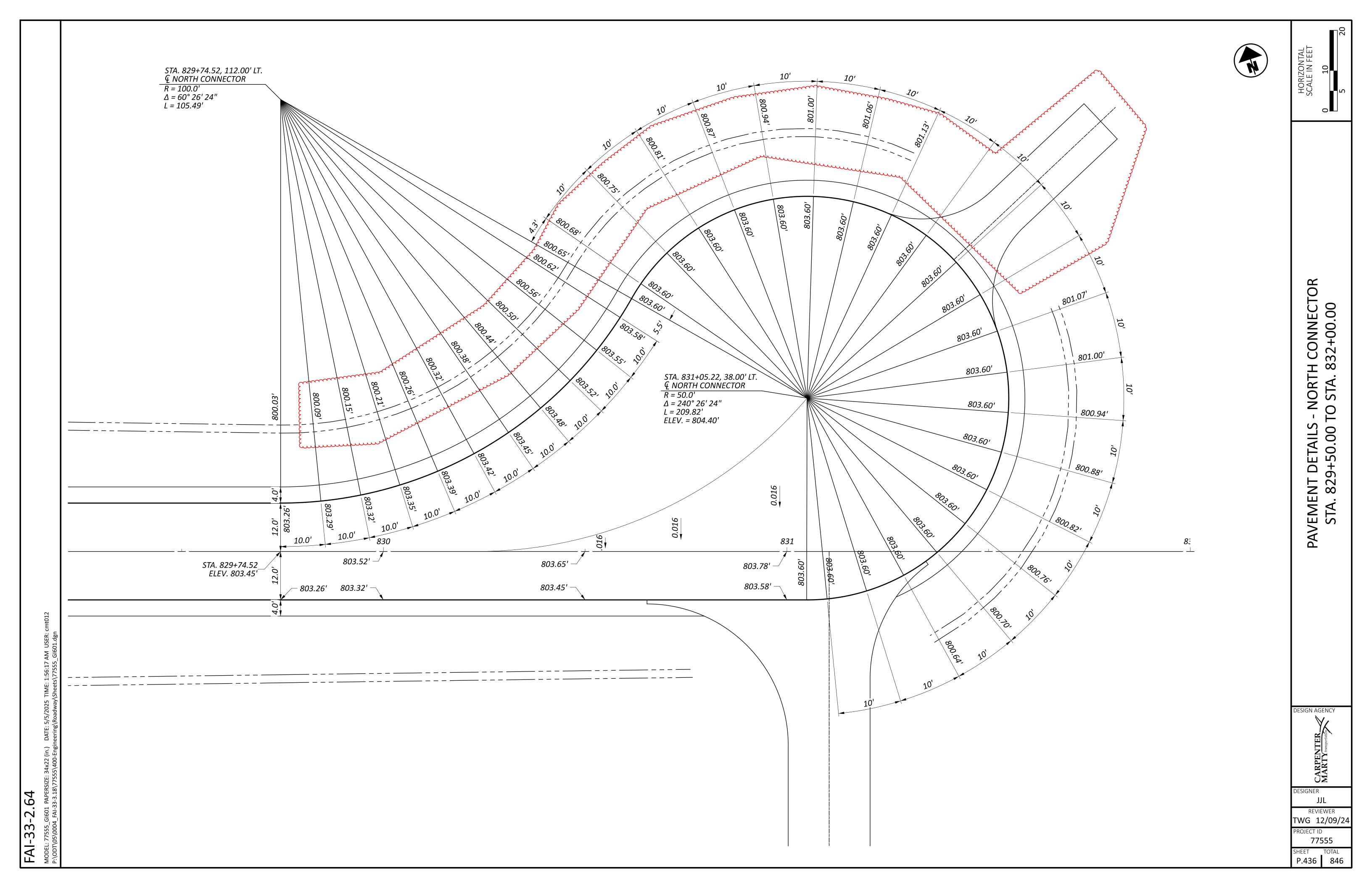




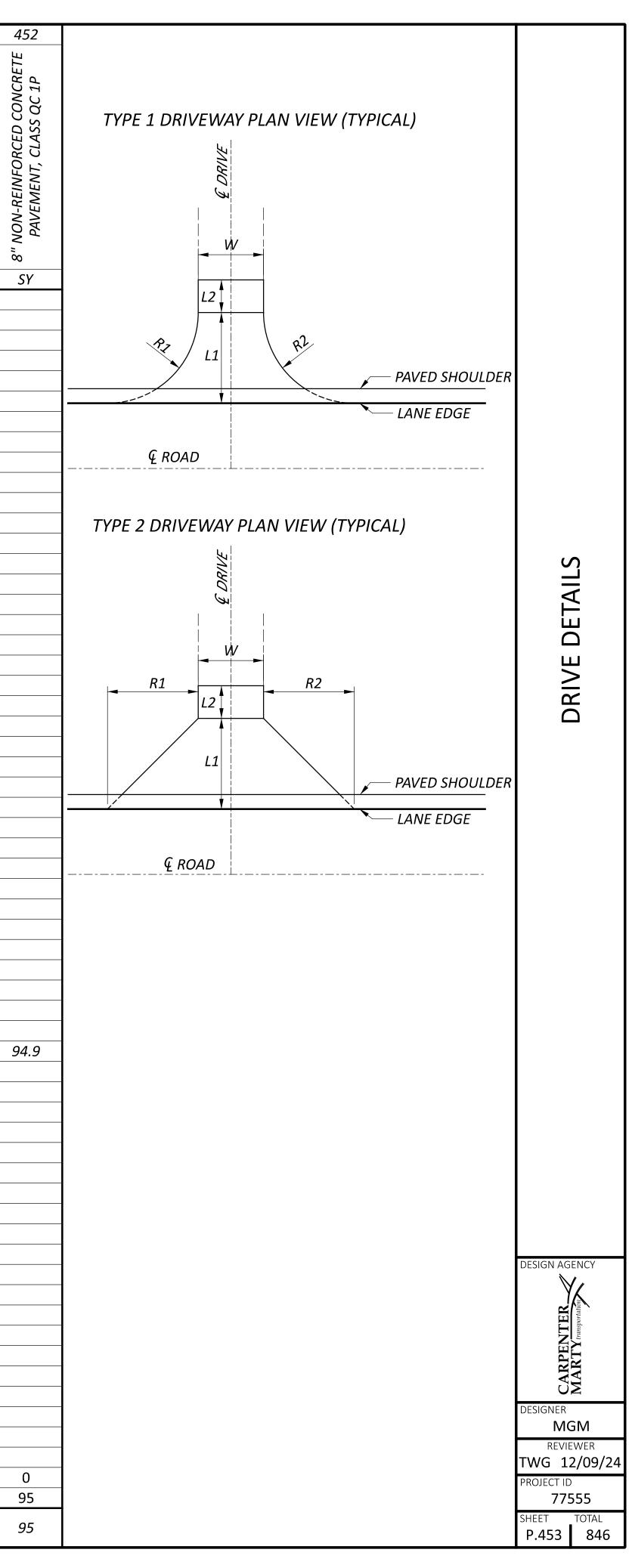
t012 dan с С USER: 55 GP : 2:39:43 AM /\Sheets\7755 TIME: 5/5/2025 DATE: : 34x22 (in.) 18\77555\40 RSIZE:

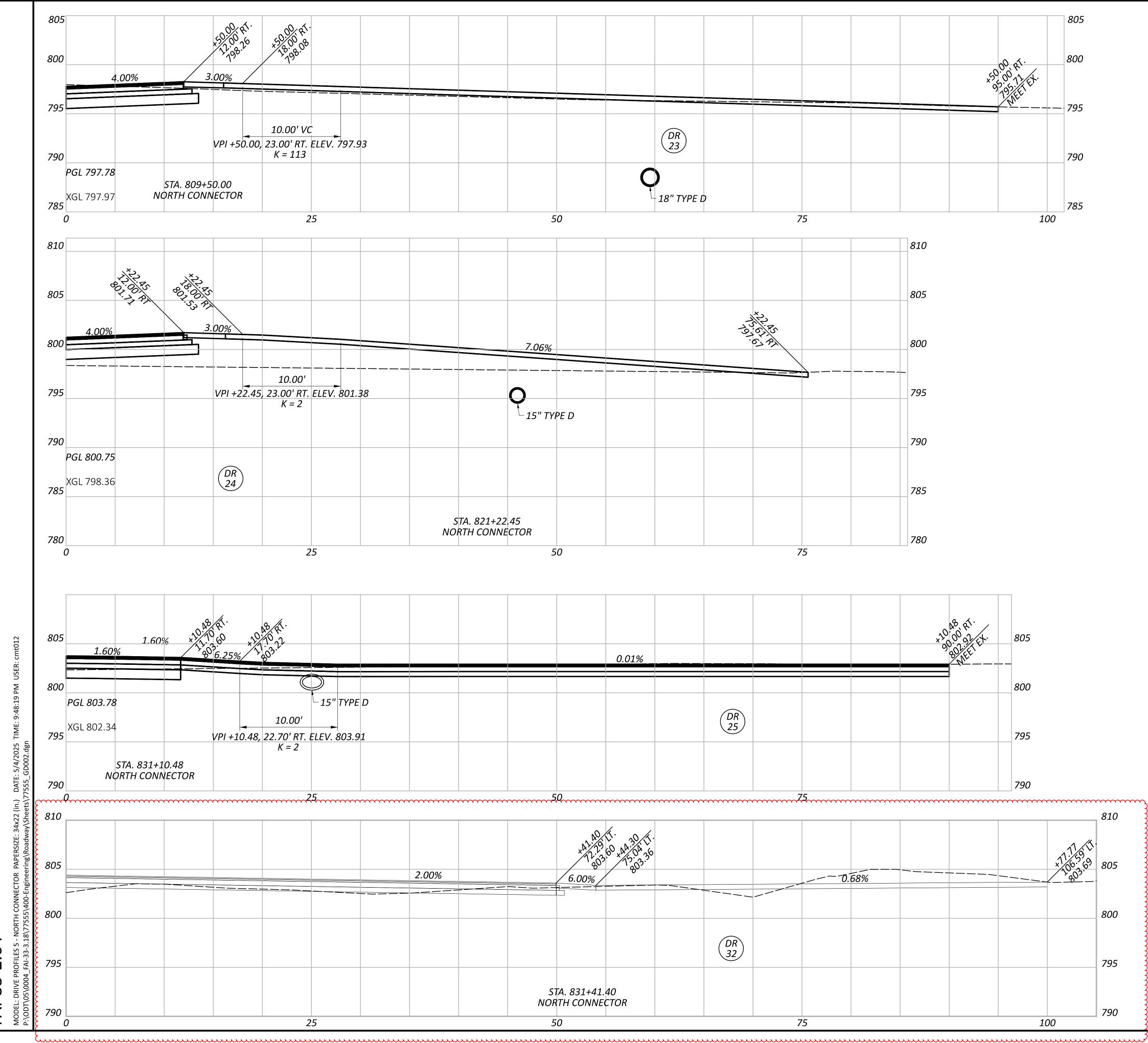


DESIGN AG	ENCY				
CARPENTER MARTY transportation					
DESIGNER					
JJL					
REVIEWER					
TWG 12/09/24					
PROJECT ID					
775	555				
SHEET	TOTAL				
P. 3 80	846				



P.324 P.324 P.324 P.324 P.324 P.327 P.357 P.357 P.357 P.357 P.357 P.357 P.359 P.359 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399 P.399	P.324 P.324 P.324 P.324 P.357 P.357 P.357 P.357 P.357 P.357 P.358 P.359 P.359 P.391 P.391 P.391 P.399 P.399 P.399	P.324 P.324 P.324 P.324 P.357 P.359 P.391 P.391 P.399 P.399 P.399 P.399 P.399	P.324 P.324 P.324 P.357 P.357 P.357 P.357 P.357 P.357 P.358 P.359	P.324 P.324 P.324	P.324 P.324		P.322 P.322 P.322 P.324	P.222 P.322	P.221 P.221 P.221 P.222 P.222 P.222 P.222 P.222	P.219	SHEET NO.
DR-20 DR-21 DR-22 DR-23 DR-24 DR-25 DR DR-26 DR-27 DR DR-28 DR-29 DR-30 DR-31	DR-20 DR-21 DR-22 DR-23 DR-24 DR-25 DR DR-26 DR-27 DR DR-28 DR-29 DR-30 DR-31	DR-20 DR-21 DR-22 DR-23 DR-24 DR-25 DR DR-25 DR DR-26 DR-27 DR DR-28 DR-29 DR-30	DR-20 DR-21 DR-22 DR-23 DR-24 DR-25	DR-19	UK-18	DR-16 DR-17	DR-12 DR-13 DR-14		DR-2 DR-3 DR-4 DR-5 DR-6 DR-7 DR-8	DR-1	REFERENCE NO.
SERVICE ROAD 1 SERVICE ROAD 2 SERVICE ROAD 2 SERVICE ROAD 2 SERVICE ROAD 2	NORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORSERVICE ROAD 1SERVICE ROAD 1SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2NORTHERN CONNECTORNORTHERN CONNECTOR	NORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORSERVICE ROAD 1SERVICE ROAD 1SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2SERVICE ROAD 2	NORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTORNORTHERN CONNECTOR	NORTHERN CONNECTOR	SOUTHERN CONNECTOR	SOUTHERN CONNECTOR SOUTHERN CONNECTOR SOUTHERN CONNECTOR	SOUTHERN CONNECTOR SOUTHERN CONNECTOR SOUTHERN CONNECTOR SOUTHERN CONNECTOR	PICKERINGTON RD SOUTHERN CONNECTOR	PICKERINGTON RDPICKERINGTON RDPICKERINGTON RDPICKERINGTON RDPICKERINGTON RDPICKERINGTON RDPICKERINGTON RDPICKERINGTON RD	PICKERINGTON RD	ROUTE
13+25.56 14+43.33 23+74.78 26+16.36 28+00.00 308+31.48	804+50.00 809+50.00 809+50.00 821+22.45 831+10.48 13+25.56 14+43.33 23+74.78 26+16.36 28+00.00 308+31.48 831+41.40	804+50.00 809+50.00 809+50.00 821+22.45 831+10.48 13+25.56 14+43.33 23+74.78 26+16.36 28+00.00	804+50.00 804+50.00 809+50.00 809+50.00 821+22.45 831+10.48	802+02.15	741+25.00	734+69.57 738+50.00 738+75.00	710+25.00 714+25.00 715+71.19 734+69.43	276+91.23 709+21.45	266+89.48 268+86.20 269+48.67 272+51.61 273+07.01 273+80.89 276+48.24	245+54.26	STATION
LT	LT RT RT RT RT LT LT LT LT LT LT LT	LT RT RT RT RT LT LT LT LT LT LT	LT RT LT RT RT RT	17		RT RT LT	RT LT LT	RT RT LT	RT LT RT LT RT LT LT	RT.	SIDE
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24.20 25.36 25.00 25.60 25.86 25.00 28.79 - - 21.00 21.00 21.00 14.23 5.30 18.29	25.36 25.00 25.60 25.86 25.00 28.79 - - - 21.00 21.00 14.23 5.30 18.29	25.36 25.00 25.60 25.86 25.00 28.79 - - - 21.00 21.00 21.00 14.23	25.36 25.00 25.60 25.86 25.00 28.79	24 20	16.33	15.00 25.00 25.00	25.00 25.10 24.89 25.00	16.00 26.00	16.00 21.00 21.00 - 26.00 23.00 21.00	43.69	H APRON LENGTH "L1"
109.51 27.83 28.00 52.40 57.14 38.72 49.51 - - 32.33 38.80 156.75 154.68 31.71	27.83 28.00 52.40 57.14 38.72 49.51 - - - 32.33 38.80 156.75 154.68 31.71	27.83 28.00 52.40 57.14 38.72 49.51 - - - 32.33 38.80 156.75	27.83 28.00 52.40 57.14 38.72 49.51	109.51	74.67	33.00 18.00 8.00	18.00 42.90 26.11 18.00	25.25 18.00	29.53 19.26 9.46 - 7.00 13.00 33.00	257.91	H DRIVEWAY LENGTH "L2"
12.00 12.00 12.00 12.00 20.30 12.00 20.30 12.00 23.39 12.00 14.00 12.00 12.00	12.00 12.00 12.00 12.00 20.30 12.00 23.39 12.00 14.00 12.00 12.00	12.00 12.00 12.00 12.00 20.30 12.00 23.39 12.00 12.00 14.00 12.00	12.00 12.00 12.00 12.00 12.00 20.30	12 00	20.00	12.00 12.00 12.00	12.00 12.00 20.55 12.00	12.00 35.26	14.14 12.00 24.24 13.08 12.38 12.00 12.00	20.00	"M" HIDIM
25.00 25.14 25.14 25.00 25.00 35.00 35.00 20.00 20.00 15.00 25.00 25.00	25.14 25.14 25.00 25.00 35.00 20.00 35.00 20.00 15.00 5.00 25.00	25.14 25.14 25.00 25.00 35.00 20.00 35.00 20.00 20.00 15.00	25.14 25.14 25.00 25.00 35.00	25.00	25.00	15.00 25.00 25.00	25.00 25.00 25.00 25.00	15.00 25.00	15.00 20.00 25.00 25.00 22.00 20.00	30.00	고 R1 (LEFT SIDE APRON WIDTH)
20.00 35.00 20.00 20.00 15.00 5.00	25.14 25.14 25.00 25.00 35.00 35.00 20.00 20.00 15.00 5.00	25.14 25.14 25.00 25.00 35.00 20.00 35.00 20.00 20.00 15.00	25.14 25.14 25.14 25.00 25.00 35.00	25.00	25.00	15.00 25.00 25.00	25.00 25.00 25.00 25.00	15.00 25.00	15.00 20.00 20.00 17.00 25.00 22.00 20.00	40.00	R2 (RIGHT SIDE APRON WIDTH
747.00 2001.00 2001.00 854.00 1045.00 2195.00 1931.00	907.00 907.00 1190.00 1286.00 1025.00 1972.00 1972.00 2001.00 2001.00 1045.00 2195.00 1931.00	907.00 907.00 1190.00 1286.00 1025.00 1972.00 1972.00 2001.00 2001.00 854.00 1045.00 2195.00	907.00 907.00 1190.00 1286.00 1025.00 1972.00	1813.00	1921.00	673.00 784.00 664.00	784.00 1077.00 1260.00 784.00	622.00 1867.00	638.00 628.00 955.00 660.00 690.00 690.00 732.00	6637.00	S CADD GENERATED SURFACE AREA
04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04	04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 01/NHS/01	04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04	04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04	04/STR/04	04/STR/04	04/STR/04 04/STR/04 04/STR/04	04/STR/04 04/STR/04 04/STR/04 04/STR/04	04/STR/04 04/STR/04	04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04 04/STR/04	04/STR/04	PART
83.0 222.3 94.9 116.1 243.9 214.6 79.7	100.8 100.8 132.2 142.9 113.9 219.1 83.0 222.3 94.9 116.1 243.9 214.6 79.7	100.8 100.8 132.2 142.9 113.9 219.1 83.0 222.3 94.9 116.1 243.9	100.8 100.8 132.2 142.9 113.9 219.1	201.4	213.4	74.8 87.1 73.8	87.1 119.7 140.0 87.1	69.1 	70.9 69.8 106.1 73.3 76.7 76.7 81.3	737.4	204 SUBGRADE COMPACTION
19.6 21.3 11.3 23.7	21.3 21.3 11.3 23.7	21.3		196			8.5	6.7 20.2	6.9 6.8 10.3 7.1 7.5 7.5		S.5" ASPHALT CONCRETE BASE, W PG64-22, (449), (DRIVEWAYS)
		21.3	21.3				8.5				S 3.5" AGGREGATE BASE
11.1 12.1 12.2 6.4 13.4	12.1 12.2 6.4 13.4	12.1 12.2 6.4		11.1			4.8	3.8 	3.9 3.8 5.8 4.0 4.2 4.2	40.6	407 NON-TRACKING TACK COAT
13.3	16.8 22.0 23.8 19.0	16.8 22.0 23.8 19.0	16.8 22.0 23.8		35.6	14.5 12.3	14.5 19.9 23.3	14.5			С 6" STABILIZED CRUSHED AGGREGATE
	49.4 47.7	49.4	18.4			16.6				163.9	A B" STABILIZED CRUSHED AGGREGATE
7.0	7.6	7.6		7 ()			3.0	7.2	2.5 2.4 3.7 2.5 2.7 2.7 2.7	25.6	Q 1.25" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, (449), 타 (DRIVEWAYS)
			10.7				4.2			35.8	1.75" ASPHALT CONCRETERintermediate course, TYPE 2, to the top (449), (DRIVEWAYS)
									81.3		6" REINFORCED CONCRETE λ PAVEMENT, CLASS QC 1P
				l							





NORTH CONNECTOR DRIVEWAY PROFILES
DESIGN AGENCY WHINGSHIFT UNITED VIENTIAL PROJECT ID 77555 SHEET TOTAL P.466 846

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWING:

SBR-1-20	REVISED	7-19-2024
VPF-1-24	DATED	7-19-2024

AND THE FOLLOWING SUPPLEMENTAL SPECIFICATION:

SS840 *REVISED* 7-19-2024

DESIGN SPECIFICATIONS

THIS STRUCTURE CONFORMS TO THE 9th EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS. 2020 AND THE ODOT BRIDGE DESIGN MANUAL, 2020.

DESIGN DATA

CONCRETE CLASS QC1 WITH QC/QA - COMPRESSIVE STRENGTH 4.0 KSI (COPING, FOOTING, MOMENT SLAB)

CONCRETE CLASS QC2 WITH QC/QA - COMPRESSIVE STRENGTH 4.5 KSI (RAILING)

CONCRETE REINFORCEMENT:

GALVANIZED STEEL REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI (WALLS, MOMENT SLABS, FOOTINGS)

GFRP REINFORCEMENT (PARAPET)

STEEL CIP PILES - ASTM A252 GRADE 3 - YIELD STRENGTH 45 KSI

OPERATIONAL IMPORTANCE

A LOAD MODIFIER OF 1.0 HAS BEEN ASSUMED FOR THE DESIGN OF THIS STRUCTURE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. ARTICLE 1.3.5 AND THE ODOT BRIDGE DESIGN MANUAL.

CONSTRUCTION CLEARANCE

MAINTAIN A CONSTRUCTION CLEARANCE OF 14 FEET HORIZONTALLY FROM THE CENTER OF THE TRACKS AND 22 FEET VERTICALLY FROM A POINT LEVEL WITH THE TOP OF THE HIGHER RAIL. AND 6 FEET FROM THE CENTER OF THE TRACKS AT ALL TIMES.

DESIGN SUBMITTALS

THE CONTRACTOR IS HEREBY NOTIFIED THAT THE RETAINING WALL SHALL BE DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS PROVIDED IN THESE NOTES. AFTER THE AWARD OF THE CONTRACT, THE CONTRACTOR SHALL SUBMIT RETAINING WALL DETAIL DESIGN PLANS (4 SETS), DESIGN CALCULATIONS (2 SETS), AND SHOP DRAWINGS PER 501.04 TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL BY THE DIRECTOR. THE PLANS SHALL BE SUBMITTED EIGHT WEEKS PRIOR TO THE BEGINNING OF CONSTRUCTION OF THE WALLS AND THE CONTRACTOR SHALL ALLOW FOUR WEEKS FOR THE REVIEW BY ODOT.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE):

THE ULTIMATE BEARING VALUE IS 55 KIPS PER PILE FOR THE PILES SUPPORTING WALLS 3 AND 4 IN FOOTING SEGMENTS 27, 28, AND 41.

WALL FOOTING PILES:

12" CAST-IN-PLACE REINFORCED CONCRETE PILES 25 FEET LONG, ORDER LENGTH

1 DYNAMIC LOAD TESTING ITEMS

PROVIDE PLAIN CYLINDRICAL CASINGS WITH A MINIMUM PILE WALL THICKNESS OF 0.250 INCHES FOR THE CAST-IN-PLACE REINFORCED CONCRETE PILES.

ITEM 203, SPECIAL - ENGINEERED FILL: LOW DENSITY CELLULAR CONCRETE FILL, CLASS II & ITEM 203, SPECIAL - ENGINEERED FILL: LOW DENSITY CELLULAR CONCRETE FILL, CLASS III

IN ADDITION TO THE REQUIREMENTS LISTED IN SPECIAL PROVISION: LOW DENSITY CELLULAR CONCRETE FILL, THE FOLLOWING **REQUIREMENTS SHALL BE MET:**

A. MATERIALS

1. ADMIXTURES

701.10 MICRO-SILICA, 701 GGBF SLAG, OR FLY ASH SHALL BE CLASS C OR CLASS F AND COMPATIBLE WITH FOAMING AGENT.

B. CONSTRUCTION METHODS 1. PLACEMENT

TOP OF THE CLASS III CCF SHALL NOT BE LESS THAN 2'-0" BELOW THE TOP OF PAVEMENT.

DO NOT PLACE CCF INTO AN AREA OF STANDING WATER.

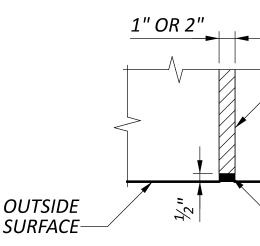
DO NOT PLACE REINFORCEMENTS AT COLD JOINTS. SUPPORT REINFORCEMENTS IN A LEVEL POSITION THROUGHOUT THEIR LENGTH AND KEEP THEM AT LEAST 6 INCHES ABOVE THE PREVIOUS DAY'S COLD JOINT.

ITEM 511 - CONCRETE, MISC: WALL SLAB

PLACE 6" THICK SLAB OF UNREINFORCED CLASS QC1 CONCRETE AS SHOWN ON SHEET 35/41 . ONCE CONCRETE HAS CURED ENOUGH TO PLACE SAWCUTTING EQUIPMENT, GROOVE SLAB WITH 2" DEEP GROOVING TOOL AT THE CONTRACTION JOINT LOCATIONS DETAILED IN THE PLAN. ALL CONCRETE, SAWCUTTING, LABOR, AND INCIDENTALS SHALL BE INCLUDED IN THE CONTRACT BID PRICE FOR ITEM 511 -CONCRETE, MISC: WALL SLAB (CY).

ITEM 516 - 1" PREFORMED EXPANSION JOINT FILLER, AS PER PLAN AND ITEM 516 - 2" PREFORMED EXPANSION JOINT FILLER, AS PER PLAN ALL 1"AND 2" P.E.J.F., AS PER PLAN CALLED FOR IN THE PLANS SHALL BE PREFORMED CORK JOINT FILLER (IN ACCORDANCE WITH ARTICLE 705.03). RECESS JOINT FILLER 1/2" FOR ALL JOINTS (SEE DETAIL). SEAL ALL JOINTS THAT ARE ABOVE GRADE WITH DECK-O-SEAL GUN GRADE-JOINT SEALANT OR AN APPROVED EQUAL. THE COLOR SHALL BE STONE GRAY. APPROVE MANUFACTURER'S APPLICATION METHODS SHALL BE FOLLOWED DURING SURFACE PREPARATION AND APPLICATION FOR MAXIMUM EFFECTIVENESS.

DECK-O-SEAL P.O. BOX 397 HAMPSHIRE. IL 60140 PHONE: 800-542-7665



PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 516 - 1" PREFORMED EXPANSION JOINT FILLER, AS PER PLAN, SF, OR ITEM 516 - 2" PREFORMED EXPANSION JOINT FILLER, AS PER PLAN, SF AND SHALL INCLUDE ALL LABOR, EQUIPMENT, AND INCIDENTALS REQUIRED TO COMPLETE THE WORK DESCRIBED.

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1" OR 2" PREFORMED CORK **EXPANSION JOINT FILLER** ½" DECK-O-SEAL GUN GRADE JOINT

SEALANT OR APPROVED EQUAL. OVER PREFORMED CORK EXPANSION JOINT FILLER (IN ACCORDANCE WITH ARTICLE 705.03)

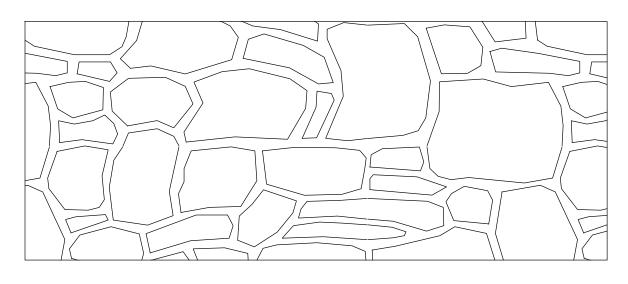
ITEM SPECIAL - 530 - STRUCTURES: AESTHETIC TREATMENT (CONCRETE FORMLINER)

THE SURFACE FINISH AESTHETICS ON THE REFERENCED PROJECT WERE CONSTRUCTED USING ONE OF THE PATTERNS DESCRIBED BELOW IN THE ARCHITECTURAL SURFACE ELEVATION AND TABLE FROM AN APPROVED COMPANY MEETING THE DETAILS SHOWN ON THIS PAGE. FOR THIS PROJECT, THE CONTRACTOR WILL RESEARCH WHICH PATTERN/COMPANY WAS USED AND, AGAIN, MATCH IT TO CONSTRUCT THIS PROJECT AS DESCRIBED BELOW.

THE SURFACE TREATMENTS REFERENCED BELOW ARE INTENDED FOR PROCEDURE, TEXTURE, AND APPEARANCE REFERENCE.

ONE FULL SCALE PATTERNED PRECONSTRUCTION TEST PANEL SHALL BE PROVIDED FOR APPROVAL BY THE DISTRICT 5 BRIDGE SECTION. IF THE TEST PANEL DOES NOT MEET THE APPROVAL OF THE DISTRICT 5 BRIDGE SECTION, THE RESULT WILL BE GROUNDS TO REJECT THE PROPOSED PANEL SURFACE CHOSEN. THE TEST PANEL WILL BE PROVIDED REPEATEDLY, AS NECESSARY, UNTIL APPROVAL IS GRANTED. THE CONTRACTOR SHALL PROVIDE AN END SECTION OF THE PARAPET, AS SHOWN IN THE PLAN, SHOWING THAT THEY CAN ACHIEVE THE FORMLINING APPLICATION AS DETAILED. THE MOCK-UP SHALL HAVE THE SAME ARCHITECTURAL RELIEF, THICKNESS, PATTERN INTENDED TO BE USED ON THE PROJECT. THE PANEL SHALL BE OF THE SAME CEMENT AND AGGREGATE SOURCE THAT WILL BE USED TO CONSTRUCT THE PROJECT. AFTER APPROVAL THE CONCRETE TEST PANEL SHALL BE REMOVED AND DISPOSED OF BY THE CONTRACTOR.

ALL AESTHETIC TREATMENT INCLUDING THE SURFACE FINISH, TEST PANELS, AND ALL OTHER MATERIALS REQUIRED TO COMPLETE THIS WORK SHALL BE INCLUDED WITH THE ITEMIZED PAYMENT FOR ITEM SPECIAL 530, STRUCTURES: AESTHETIC TREATMENT (CONCRETE FORMLINER).



ARCHITECTURAL SURFACE - ELEVATION

THE FOLLOWING SHALL BE USED:

COMPANY NAME:	PANEL SURFACE TREATMENT:	SPECIFICATIONS:
SPEC FORMLINERS, INC.	WASHINGTON DRYSTACK #1581	MAX RELIEF: 1½" LINER THICKNESS: 2⅛" STONE SIZE: 4" TO 24"
CUSTOM ROCK INTERNATIONAL	NEW ENGLAND DRYSTACK #12003	MAX RELIEF: 1 ³ / ₈ " LINER THICKNESS: 2 ¹ / ₄ " STONE SIZE: 3" TO 24"
APPROVED EQUAL	APPROVED EQUAL	APPROVED EQUAL

THE DEPARTMENT WILL PAY FOR THE ACCEPTED QUANTITIES AT THE CONTRACT PRICE FOR ITEM 607 - VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC, AS PER PLAN.

ITEM 607 - VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC, AS PER PLAN FABRICATE AND INSTALL THE VANDAL PROTECTION FENCE AS DETAILED IN THIS PLAN AND STANDARD DRAWING VPF-1-24. THE VANDAL PROTECTION

FENCE SHALL BE 6'-0" STRAIGHT FENCE. THE COATING SYSTEM USED FOR THIS FENCE SHALL BE MODIFIED AS FOLLOWS. IF NOT ALREADY SPECIFIED IN VPF-1-24, ALL STEEL COMPONENTS SHALL RECEIVE PVC COATING IN ADDITION TO THE STANDARD SURFACE TREATMENTS. ALL THREADED ASSEMBLY COMPONENTS (I.E. THREAD LENGTH OF BOLTS, NUTS, AND WASHERS) WILL BE EXCLUDED FROM THIS ADDITIONAL COATING REQUIREMENT. PVC COATINGS SHALL CONFORM TO EITHER ASTM F668 CLASS 2A OR 2B (MESH, WIRE, ETC.), ASTM F626-14 (FENCE FITTINGS, ETC.), OR ASTM F1043-16 (FRAMEWORK, POSTS, RAILS, ETC.).

DUE TO THE ADDITIONAL THICKNESS OF THIS COATING SYSTEM, THE POTENTIAL EXISTS THAT TYPICAL FITTINGS MAY REQUIRE THEIR SIZES INCREASED ABOVE THE STANDARD SIZES SHOWN IN STD. DWG. VPF-1-24. IT IS THE RESPONSIBILITY OF THE CONTRACTOR/FABRICATOR TO TEST ALL FENCE COMPONENTS FOR FIT-UP AT THE FABRICATION STAGE AND TO INCORPORATE ANY SIZE-UP ADJUSTMENTS TO ENSURE EASE OF FIELD INSTALLATION AND ERECTION. THE FINAL COLOR FOR ALL PVC COATED FENCE COMPONENTS SHALL BE BLACK (CLOSELY APPROACHING AMS 595A-17038). HANDLE ALL PVC COATED MATERIALS WITH CARE. IF THE PVC COATING IS DAMAGED. REPLACE THE DAMAGED FENCE COMPONENT(S) AT NO COST TO THE DEPARTMENT.

THE DEPARTMENT WILL MEASURE THIS WORK ON A LINEAR FEET BASIS.

S NOTE S ENERAL WA J VARIES ESIGN AGENCY CARPENTER MARTY transportati ESIGNER CHECKER BWR JMV REVIEWER GDJ 10-18-23 ROJECT ID 77555 JBSET TOTAL 3 41 HEET TOTAL P.505 846

ITEM 530 - SPECIAL - STRUCTURES: PRECAST WALL PANELS A. DESCRIPTION THIS BID ITEM CONSISTS OF PRECAST WALLS MANUFACTURED AND CONSTRUCTED IN ACCORDANCE WITH THIS SPECIFICATION AND DESIGNED IN ACCORDANCE WITH THE 9th EDITION OF THE	THE FORMS USED IN MA RIGID AND ACCURATE T WITHIN THE PERMISSIB CASTING SURFACE SHAL THE WALL SECTION SHA
"LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2020 AND THE ODOT BRIDGE DESIGN MANUAL, 2020.	PREVENT CRACKING OR THE FRONT FACE OF THI HAVE AN AESTHETIC FIN BETWEEN PRECAST PAN
PRECAST WALL PANELS ARE SHOWN AND DETAILED IN THE PLANS WITH A 10 INCH STRUCTURAL THICKNESS AND A 2 INCH AESTHETIC THICKNESS. OTHER STRUCTURAL WALL THICKNESSES ARE ACCEPTABLE PROVIDING THEY MEET ALL THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. ANY	PLAN DETAILS. THE BACK PANELS SHALL HAVE A U ROUGH SCREEDED TO EN AND SURFACE DISTORTIN
ADDITIONAL MATERIAL REQUIRED TO ACCOMMODATE ALTERNATE WALL DIMENSIONS SHALL BE AT THE CONTRACTOR'S EXPENSE.	ALL PANELS SHALL BE M DIMENSIONS WITHIN 1/
B. DESIGN DATA CONCRETE - COMPRESSIVE STRESS 4 KSI	H. COMPRESSIVE STRENGTH ACCEPTANCE OF THE CO COMPRESSIVE STRENGT
CONCRETE REINFORCEMENT: - GALVANIZED STEEL REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI	PRODUCTION LOTS. A PI PANELS THAT WILL BE RI STRENGTH SAMPLE AND SINGLE DAY'S PRODUCT
WELDED WIRE FABRIC - MINIMUM YIELD STRENGTH 70 KSI	DURING THE PRODUCTI
C. MATERIALS - CONCRETE THE CONCRETE FOR THE WALL SECTIONS SHALL BE COMPOSED OF PORTLAND CEMENT, FINE AND COARSE AGGREGATES, ADMIXTURES AND WATER. PORTLAND CEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM SPECIFICATION C150, TYPE I, II,	MANUFACTURER WILL F ACCORDANCE WITH AST SAMPLE, CONSISTING O RANDOMLY SELECTED F
OR III. THE AIR ENTRAINING ADMIXTURE SHALL CONTAIN 6% ± 2% ENTRAINED AIR, AND SLUMP SHALL BE MAINTAINED WITH THE RANGE OF 1" TO 4". THE SLUMP MAY BE INCREASED TO 7" PROVIDED THE INCREASE IS ACHIEVED BY THE ADDITION OF A	CYLINDERS FOR COMPR 1'-0" SPECIMENS PREPA EVERY COMPRESSIBLE S CYLINDERS WILL BE CUR
CHEMICAL WATER-REDUCING ADMIXTURE APPROVED BY THE ENGINEER.	AND TESTED AT APPROX COMPRESSIVE STRENGT ACCORDANCE WITH AST
D. MATERIALS AND REINFORCING HARDWARE REINFORCEMENT SHALL CONSIST OF WELDED WIRE FABRIC CONFORMING TO ASTM A185, OR DEFORMED BILLET-STEEL BARS CONFORMING TO ASTM A615, A616, OR A617, GRADE 60.	WILL DETERMINE THE IN ADDITION, 2 CYLINDERS ASTM C31 AND TESTED STRENGTH OF THESE TW
E. SHOP DRAWING REQUIREMENTS THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO MANUFACTURE. THE SHOP	ACCORDANCE WITH AST STRENGTH TEST RESULT STRENGTH OF THE PROL IF THE INITIAL STRENGT
<i>DRAWINGS SHALL INCLUDE THE FOLLOWING. - ALL STRUCTURAL DESIGN AND LOADING INFORMATION - A PLAN VIEW - ALL ELEVATION VIEWS - ALL DIMENSIONS</i>	STRENGTH IN EXCESS OF UTILIZED AS THE COMPI PRODUCTION LOT AND WILL BE WAIVED FOR TH
MANUFACTURING SHALL NOT BEGIN UNTIL WRITTEN ACCEPTANCE OF THE SUBMITTED SHOP DRAWINGS HAS BEEN RECEIVED.	ACCEPTANCE OF A PROL COMPRESSIVE STRENGT TO 4 KSI. IF THE RESULT PRODUCTION LOT WILL
F. TESTING AND INSPECTION ACCEPTABILITY OF THE CONCRETE FOR THE PRECAST PANELS WILL BE DETERMINED ON THE BASIS OF COMPRESSION TESTS, CERTIFICATIONS AND VISUAL INSPECTIONS. THE CONCRETE STRENGTH REQUIREMENTS FOR THE PRECAST PANELS SHALL BE CONSIDERED ATTAINED REGARDLESS OF CURING AGE WHEN COMPRESSION TEST RESULTS INDICATE STRENGTH WILL	<i>THREE ACCEPTANCE CRI - 90% OF THE CON OVERALL PRODU - THE AVERAGE OI STRENGTH TEST - NO INDIVIDUAL FALL BELOW 3.6</i>
COMPRESSION TEST RESOLTS INDICATE STRENGTH WILL CONFORM TO 28-DAY SPECIFICATIONS AS STATED BELOW. THE MANUFACTURER SHALL FURNISH FACILITIES AND PERFORM ALL NECESSARY SAMPLING AND TESTING IN AN EXPEDITIOUS AND SATISFACTORY MANNER. PANELS UTILIZING TYPE I OR TYPE II CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL WHEN 7-DAY INITIAL STRENGTHS EXCEED 85 PERCENT	IN THE EVENT THAT A PI SPECIFIED COMPRESSIV PRODUCTION LOT SHAL PREVAIL UNLESS THE M OBTAINS AND SUBMITS
OF THE 28-DAY REQUIREMENTS. PANELS UTILIZING TYPE III CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL PRIOR TO 28 DAYS ONLY WHEN COMPRESSIVE STRENGTH TEST RESULTS INDICATE THAT THE STRENGTH EXCEEDS THE 28-DAY SPECIFICATION.	ENGINEER THAT THE STI PLACED WITHIN THE PA ACCEPTABLE. IF SUCH EN TAKEN FROM THE PANE SHALL BE OBTAINED AN SPECIFICATIONS OF ASTI
G. MANUFACTURE THE AGGREGATES, CEMENT AND WATER SHALL BE PROPORTIONED AND MIXED IN A BATCH MIXER TO PRODUCE A HOMOGENEOUS CONCRETE MEETING THE STRENGTH REQUIREMENTS OF THESE NOTES. THE PROPORTION OF PORTLAND CEMENT IN THE MIXTURE SHALL NOT BE LESS THAN 564 POUNDS PER CUBIC YARD OF CONCRETE.	
THE WALL SECTIONS SHALL BE CURED FOR A SUFFICIENT LENGTH OF TIME SO THAT THE CONCRETE WILL DEVELOP THE SPECIFIED COMPRESSIVE STRENGTH IN 28 DAYS OR LESS. ANY ONE OF THE METHODS OF CURING OR COMBINATION THEREOF SHALL BE USED:	
STEAM CURING - THE SECTIONS MAY BE LOW PRESSURE, STEAM CURED BY A SYSTEM THAT WILL MAINTAIN A MOIST ATMOSPHERE.	
WATER CURING - THE SECTIONS MAY BE WATER CURED BY ANY METHOD THAT WILL KEEP THE SECTIONS MOIST.	

USED IN MANUFACTURE SHALL BE SUFFICIENTLY CCURATE TO MAINTAIN THE SECTIONS DIMENSIONS PERMISSIBLE VARIATIONS GIVEN THESE NOTES. ALL RFACE SHALL BE OF SMOOTH MATERIAL.

CTION SHALL BE STORED IN SUCH A MANNER TO ACKING OR DAMAGES.

ACE OF THE REINFORCED CONCRETE PANELS SHALL STHETIC FINISH AS SHOWN IN THE PLANS. CAULKING ECAST PANELS SHALL BE IN ACCORDANCE WITH THE S. THE BACK SIDE OF THE REINFORCED CONCRETE L HAVE A UNIFORM SURFACE FINISH AND SHALL BE EDED TO ELIMINATE OPEN POCKETS OF AGGREGATE E DISTORTIONS IN EXCESS OF 1/4".

SHALL BE MANUFACTURED WITH ALL PANEL S WITHIN 1/4".

OF THE CONCRETE PANELS WITH RESPECT TO TE STRENGTH WILL BE DETERMINED ON THE BASIS OF N LOTS. A PRODUCTION LOT IS DEFINED AS A GROUP OF WILL BE REPRESENTED BY A SINGLE COMPRESSIVE AMPLE AND WILL CONSIST OF EITHER 6 PANELS OR A S PRODUCTION, WHICHEVER IS LESS.

PRODUCTION OF THE CONCRETE PANELS THE IRER WILL RANDOMLY SAMPLE THE CONCRETE IN E WITH ASTM C172. A SINGLE COMPRESSIVE STRENGTH INSISTING OF A MINIMUM OF FOUR CYLINDERS, WILL BE SELECTED FOR EVERY PRODUCTION LOT.

OR COMPRESSIVE STRENGTH TESTS SHALL BE 6" DIA. X ENS PREPARED IN ACCORDANCE WITH ASTM C31. FOR PRESSIBLE STRENGTH SAMPLE, A MINIMUM OF 2 VILL BE CURED IN THE SAME MANNER AS THE PANELS AT APPROXIMATELY 7 DAYS. THE AVERAGE 'E STRENGTH OF THESE CYLINDERS WHEN TESTED IN E WITH ASTM C39, WILL PROVIDE A TEST RESULT WHICH MINE THE INITIAL STRENGTH OF THE CONCRETE. IN CYLINDERS SHALL BE CURED IN ACCORDANCE WITH ND TESTED AT 28 DAYS. THE AVERAGE COMPRESSIVE F THESE TWO CYLINDERS, WHEN TESTED IN E WITH ASTM C39, WILL PROVIDE A COMPRESSIVE EST RESULT WHICH WILL DETERMINE THE COMPRESSIVE F THE PRODUCTION LOT.

L STRENGTH TESTS RESULTS INDICATE A COMPRESSIVE I EXCESS OF 4 KSI, THEN THESE TESTS RESULTS WILL BE THE COMPRESSIVE STRENGTH TEST RESULT FOR THE I LOT AND THE REQUIREMENT FOR TESTING AT 28 DAYS VED FOR THAT PARTICULAR PRODUCTION LOT.

E OF A PRODUCTION LOT WILL BE MADE IF THE VE STRENGTH TEST RESULT IS GREATER THAN OR EQUAL THE RESULT IS LESS THAN 4 KSI, THE ACCEPTANCE OF THE N LOT WILL BE BASED ON ITS MEETING THE FOLLOWING PTANCE CRITERIA:

- OF THE COMPRESSIVE STRENGTH TEST RESULTS FOR THE ALL PRODUCTION SHALL EXCEED 4 KSI.
- AVERAGE OF ANY SIX CONSECUTIVE COMPRESSIVE
- NGTH TEST RESULTS SHALL EXCEED 4 KSI. IDIVIDUAL COMPRESSIVE STRENGTH TEST RESULT SHALL BELOW 3.6 KSI.

T THAT A PRODUCTION LOT FAILS TO MEET THE OMPRESSIVE STRENGTH REQUIREMENTS, THE N LOT SHALL BE REJECTED. SUCH REJECTION SHALL ESS THE MANUFACTURER, AT THEIR OWN EXPENSE, O SUBMITS EVIDENCE ACCEPTABLE TO THE HAT THE STRENGTH AND QUALITY OF THE CONCRETE HIN THE PANELS OF THE PRODUCTION LOT IS IF SUCH EVIDENCE CONSISTS OF TESTS MADE ON CORES ITHE PANELS WITHIN THE PRODUCTION LOT. THE CORES TAINED AND TESTED IN ACCORDANCE WITH THE ONS OF ASTM C42. I. REJECTION

PANELS SHALL BE SUBJECT TO REJECTION BECAUSE OF FAILURE TO MEET ANY OF THE REQUIREMENTS SPECIFIED ABOVE. IN ADDITION, ANY OR ALL OF THE FOLLOWING DEFECTS MAY BE SUFFICIENT CAUSE FOR REJECTION:

- DEFECTS THAT INDICATE IMPERFECT MOLDING
- DEFECTS INDICATING HONEYCOMBED OR OPEN TEXTURED CONCRETE
- DEFECTS IN THE PHYSICAL CHARACTERISTICS OF THE CONCRETE, OR DAMAGE TO THE SEALING OF CONCRETE SURFACE TREATMENT OR TO AESTHETIC SURFACE TREATMENTS
- CONCRETE CHIPS OR SPALLS THAT ARE LARGER THAN 4 INCHES WIDE OR 2 INCHES DEEP. REPAIR ALL CHIPS AND SPALLS THAT ARE SMALLER
- STAINED FORM FACES, DUE TO FORM OIL, CURING, OR OTHER CONTAMINANTS
- SIGNS OF AGGREGATE SEGREGATION
- CRACKS WIDER THAN 0.01 INCHES, PENETRATING MORE THAN 1 INCH OR LONGER THAN 20 PERCENT OF THE LENGTH OF THE FACE CONTAINING THE CRACK. REPAIR ALL CRACKS THAT ARE SMALLER
- PANELS THAT DO NOT MEET THE SPECIFIED DIMENSIONAL TOLERANCES
- UNUSABLE LIFTING INSERTS
- EXPOSED REINFORCING STEEL
- INSUFFICIENT CONCRETE COMPRESSIVE STRENGTH

EITHER REPLACE DAMAGED PRECAST WALL PANELS OR DOCUMENT THE DAMAGE AND PROPOSE TO THE ENGINEER A REPAIR METHOD FOR THE DAMAGED PANEL; PERFORM REPAIRS WITH THE ACCEPTANCE OF THE ENGINEER. PROVIDE ACCEPTABLE REPLACEMENT PANELS FOR ANY THAT ARE REJECTED.

J. MARKING

THE DATE OF MANUFACTURE, THE PRODUCTION LOT NUMBER AND THE PIECE MARK SHALL BE CLEARLY SCRIBED ON THE BACK SURFACE OF EACH PANEL.

K. CONCRETE LEVELING PAD

THE CONCRETE LEVELING PAD (MUD SLAB) SHALL BE CONSTRUCTED WITH CONCRETE HAVING A STRENGTH THAT IS NOT LESS THAN 3.5 KSI AND SHALL HAVE SUFFICIENT STRENGTH TO ADEQUATELY SUPPORT THE PANELS AT THE BOTTOM OF THE WALL IN A LEVEL POSITION DURING INSTALLATION.

A 4" (MIN.) THICK UNREINFORCED CONCRETE LEVELING PAD SHALL BE RROVIDED AS SHOWN ON THE PLANS. THE PAD SHALL BE CURED A MINIMUM OF 24 HOURS BEFORE PLACING WALL PANELS ON THE LEVELING PAD.

L. WALL ERECTION

PANELS ARE HANDLED BY MEANS OF A LIFTING DEVICE CONNECTED TO THE LIFTING INSERT WHICH IS CAST INTO THE UPPER EDGE OR BACK SIDE OF THE PANELS. ALL PANELS SHALL BE BRACED TO RESIST THE TEMPORARY CONSTRUCTION LOADS INCLUDING WIND LOADS, PRIOR TO FOOTING CONSTRUCTION.

M. BASIS OF PAYMENT

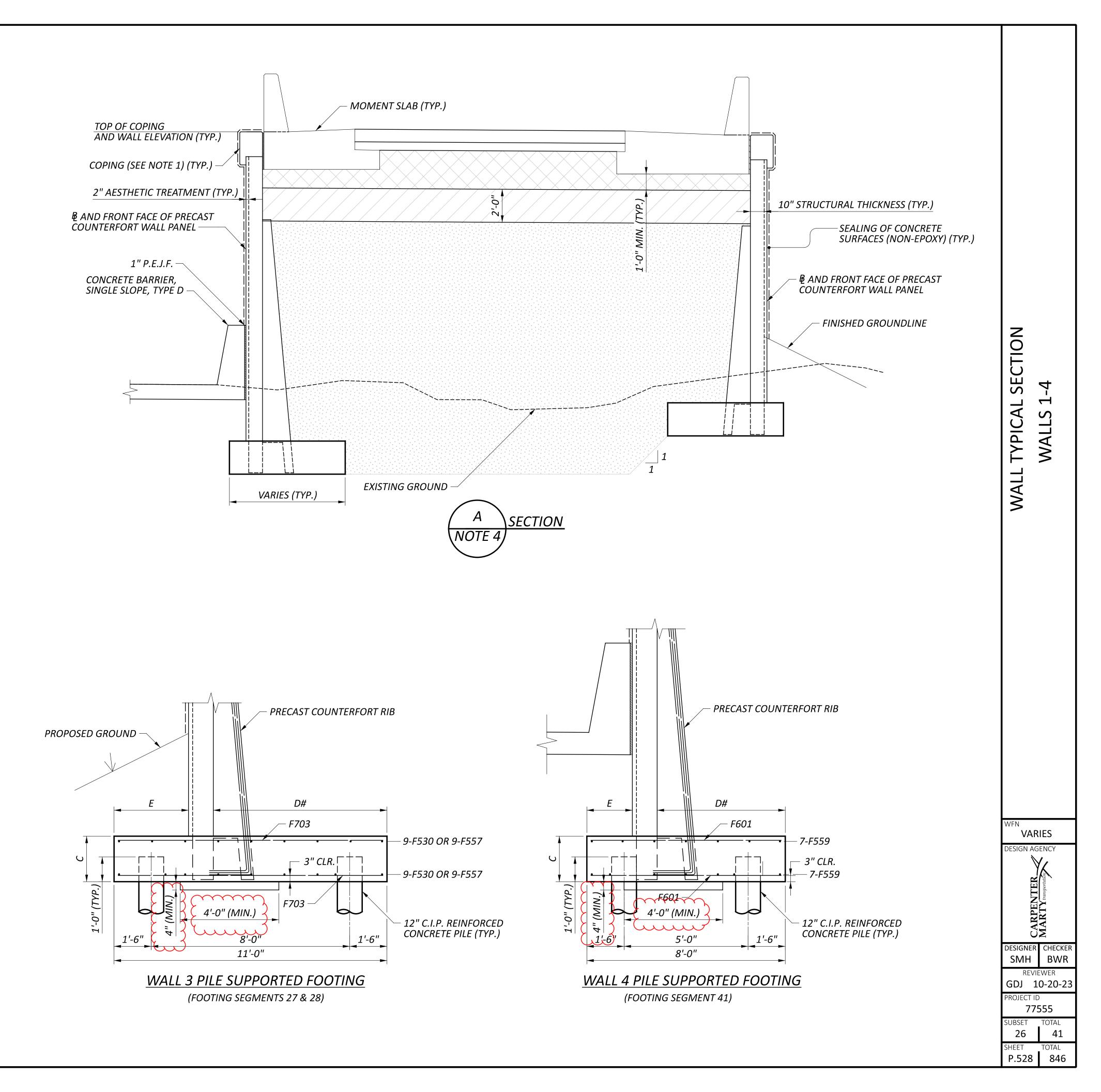
PAYMENT FOR ITEM 530- SPECIAL - STRUCTURES: PRECAST WALL PANELS COVERS ALL WORK DESCRIBED ABOVE.

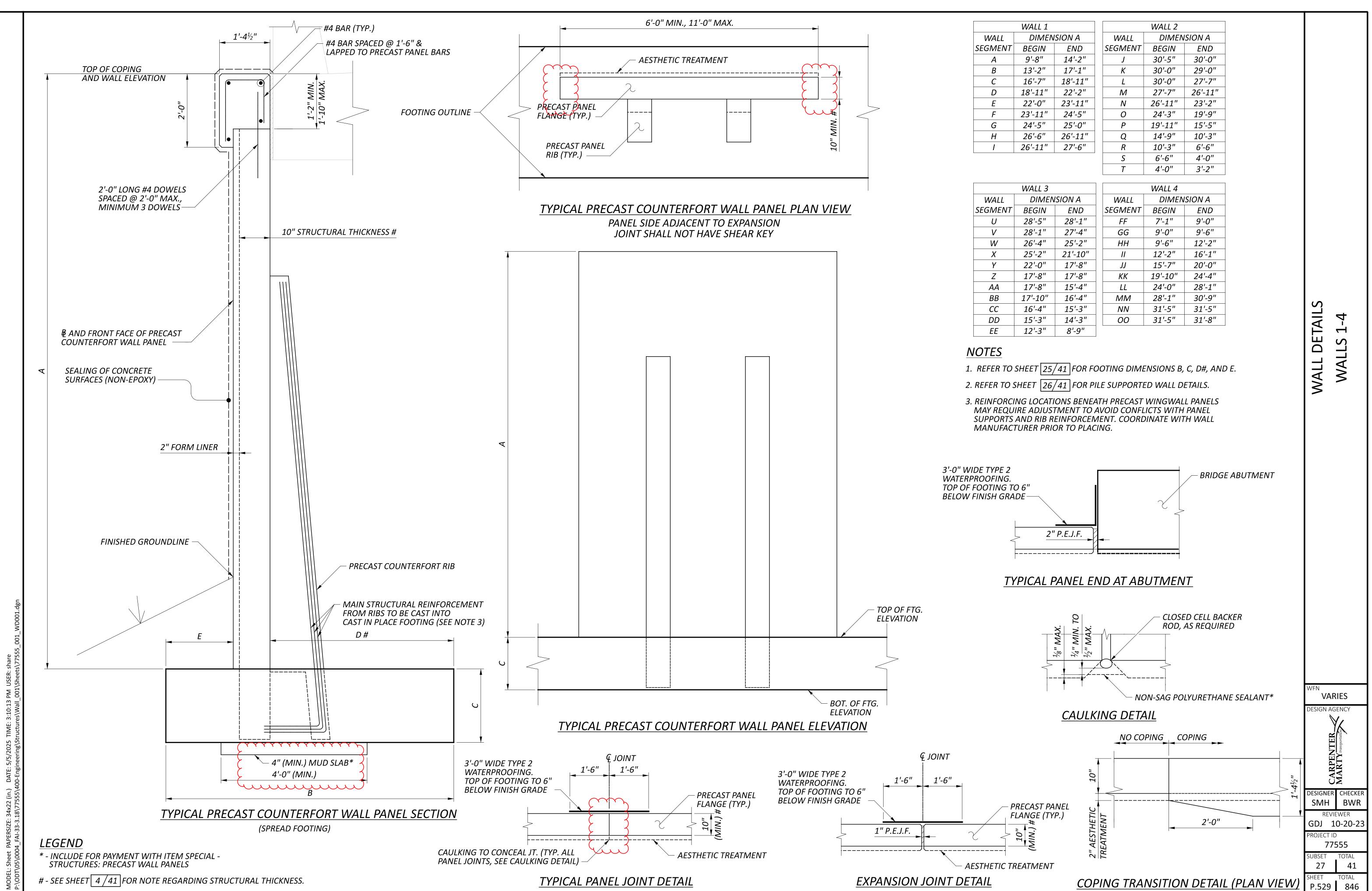
FOUNDATION BEARING RESISTANCE

PRECAST WALL PANEL FOOTINGS, AS DESIGNED, PRODUCE A MAXIMUM STRENGTH LIMIT STATE BEARING PRESSURE OF 2.27 KIPS PER SQUARE FOOT FOR WALLS 1 & 2 AND 3.34 KIPS PER SQUARE FOOT FOR WALLS 3 AND 4. THE FACTORED BEARING RESISTANCE IS 2.21 KIPS PER SQUARE FOOT FOR WALLS 1 & 2 AND 2.28 KIPS PER SQUARE FOOT FOR WALLS 3 AND 4.

GENERAL NOTES WALLS	
WFN VARIES DESIGN AGENCY	
CARPENTER	
DESIGNER CHECKER BWR JMV REVIEWER GDJ 10-18-23	
PROJECT ID 77555 SUBSET TOTAL	
4 41 SHEET TOTAL P.506 846	

	 LEGEND LIGHTWEIGHT CELLULAR CONCRETE FILL, CLASS II LIGHTWEIGHT CELLULAR CONCRETE FILL, CLASS III LIGHTWEIGHT CELLULAR CONCRETE FILL, CLASS III ITEM 203 - EMBANKMENT NOTES FOR COPING DETAILS, SEE SHEET 27/41 FOR MOMENT SLAB BARRIER DETAILS, SEE SHEET 28/41 - 34/41 ALL DIMENSIONS ARE PERPENDICULAR TO WALL. FOR SECTION A LOCATIONS SEE SHEET 6/41 - 17/41 THE 6" DIAMETER PERFORATED PERFORATED CORRUGATED PLASTIC PIPE MAY BE PLACED BEHIND THE COUNTERFORT INSTEAD OF THROUGH THE CONTRERFORT ADDITIONAL POROUS BACKFILL WITH FILTER FABRIC TO BE PROVIDED AT NO ADDITIONAL COST TO THE DEPARTMENT. SEE SHEET 25/41 FOR LOCATION OF DIMENSIONS B, C, D#, AND E. REINFORCING LOCATIONS BENEATH PRECAST WINGWALL PANELS MAY REQUIRE ADJUSTMENT TO AVOID CONFLICTS WITH PANELS SUPFORTS AND BIS REINFORCEMENT. COORDINATE WITH WALL MANUFACTURER PRIOR TO PLACING.
FAI-33-2.64 MODEL: Sheet PAPERSIZE: 34x22 (in.) DATE: 5/5/2025 TIME: 3:10:09 PM USER: share P:\ODT\05\0004_FAI-33-3.18\77555\400-Engineering\Structures\Wall_001\Sheets\7755_001_WY001.dgn	(v_{ARIES})

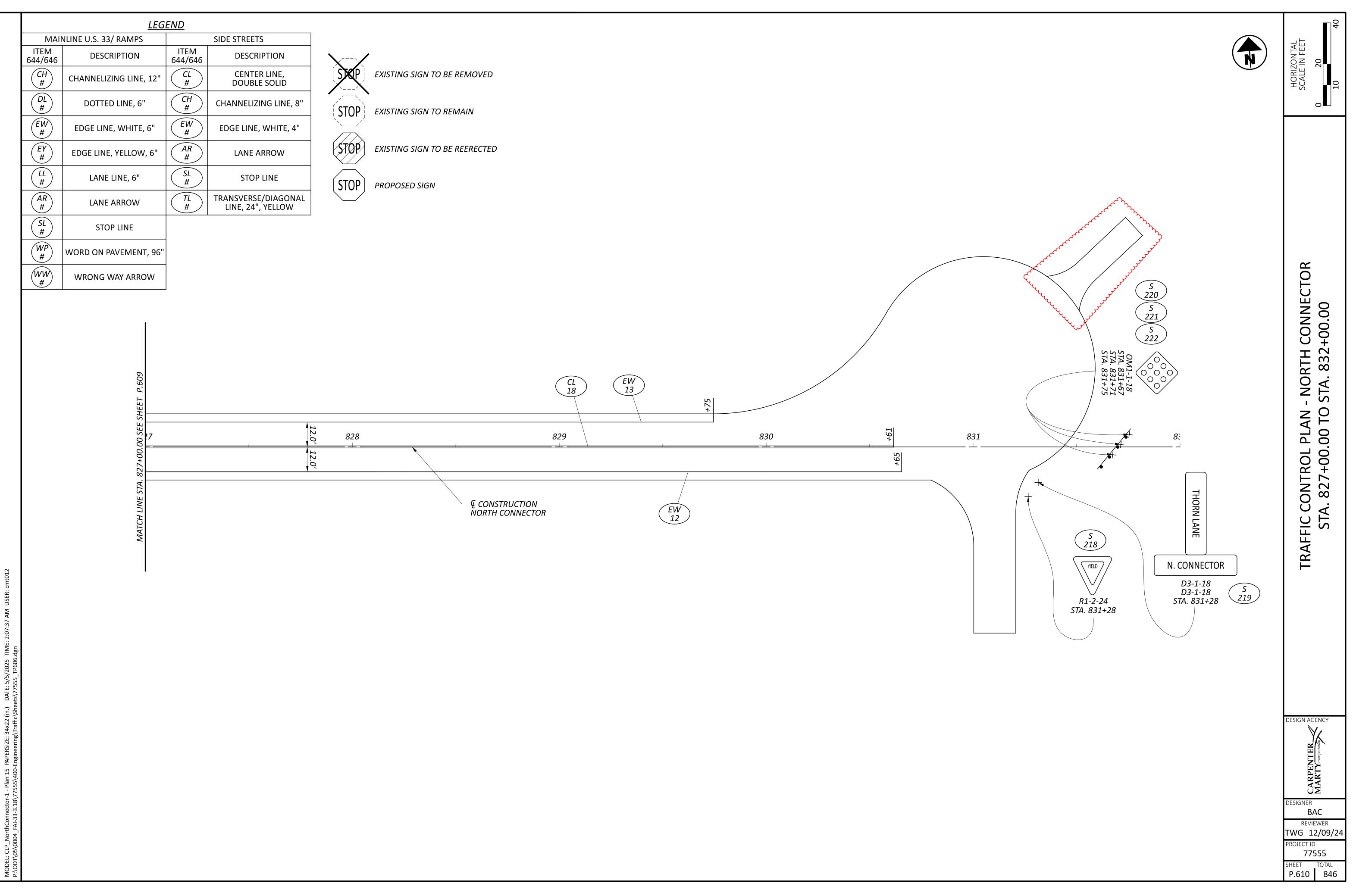


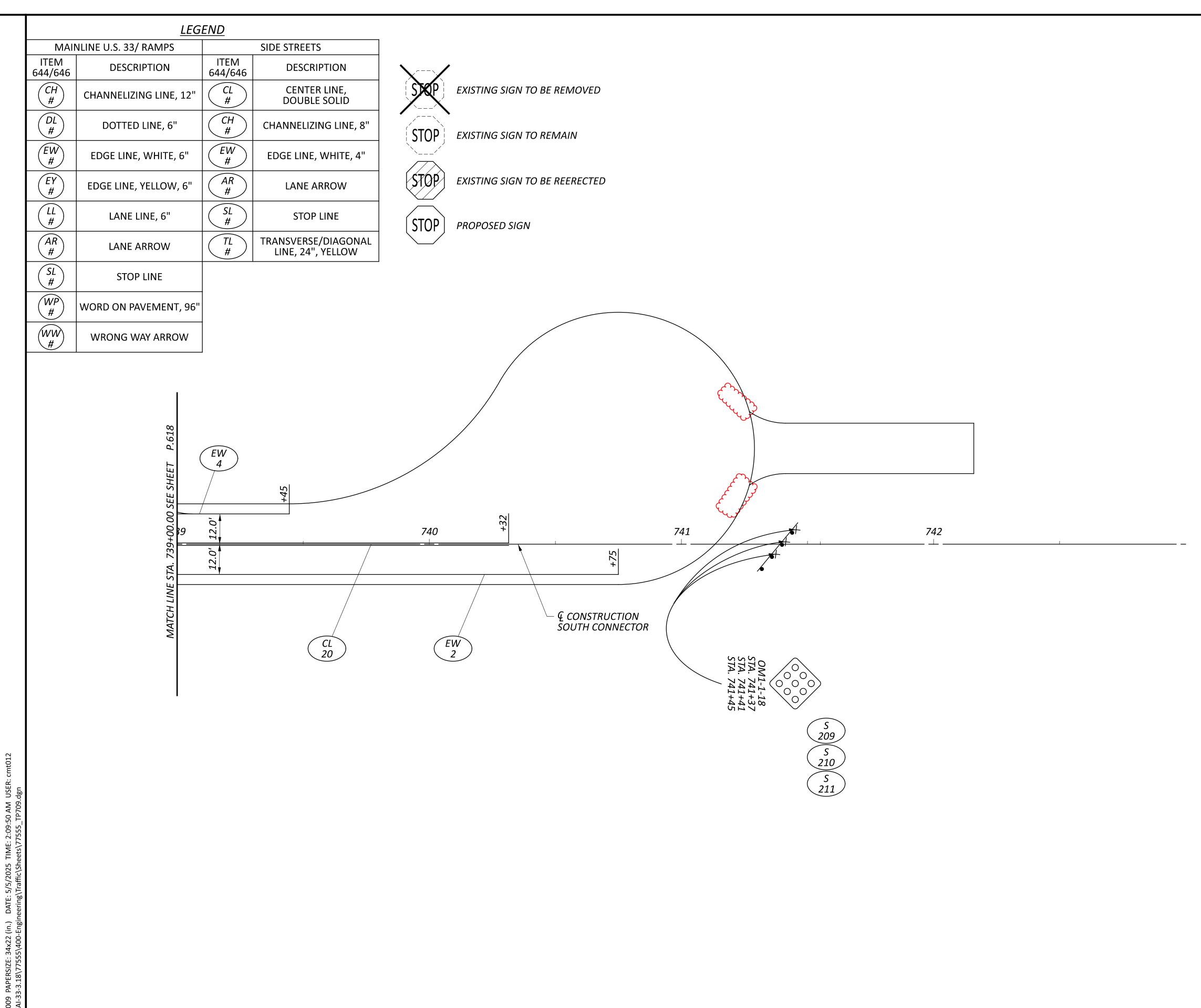


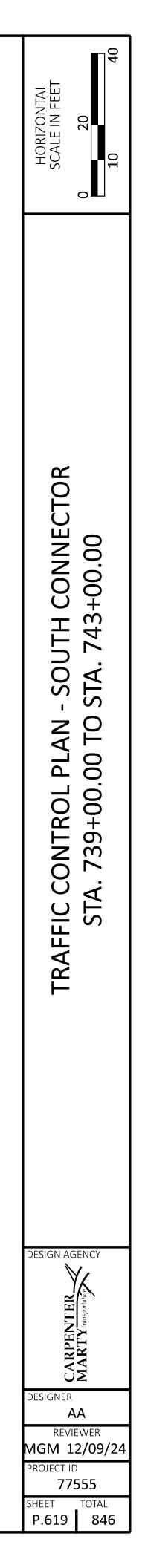
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ITEM 530 - SPECIAL - STRUCTURES: PRECAST WALL PANELS A. DESCRIPTION THIS BID ITEM CONSISTS OF PRECAST WALLS MANUFACTURED	THE FORMS USED IN MA RIGID AND ACCURATE TO WITHIN THE PERMISSIB CASTING SURFACE SHAL
AND CONSTRUCTED IN ACCORDANCE WITH THIS SPECIFICATION AND DESIGNED IN ACCORDANCE WITH THE 9th EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE	THE WALL SECTION SHA PREVENT CRACKING OR
AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2020 AND THE ODOT BRIDGE DESIGN MANUAL, 2020. PRECAST WALL PANELS ARE SHOWN AND DETAILED IN THE PLANS	THE FRONT FACE OF THE HAVE AN AESTHETIC FIN BETWEEN PRECAST PAN PLAN DETAILS. THE BAC
WITH A 10 INCH STRUCTURAL THICKNESS AND A 2 INCH AESTHETIC THICKNESS. OTHER STRUCTURAL WALL THICKNESSES ARE ACCEPTABLE PROVIDING THEY MEET ALL THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. ANY ADDITIONAL MATERIAL REQUIRED TO ACCOMMODATE	PANELS SHALL HAVE A U ROUGH SCREEDED TO E AND SURFACE DISTORTI ALL PANELS SHALL BE M
ALTERNATE WALL DIMENSIONS SHALL BE AT THE CONTRACTOR'S EXPENSE.	DIMENSIONS WITHIN 1/
B. DESIGN DATA CONCRETE - COMPRESSIVE STRESS 4 KSI	H. COMPRESSIVE STRENGTH ACCEPTANCE OF THE CO COMPRESSIVE STRENGT PRODUCTION LOTS. A PI
CONCRETE REINFORCEMENT: - GALVANIZED STEEL REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI	PANELS THAT WILL BE R STRENGTH SAMPLE AND SINGLE DAY'S PRODUCT
WELDED WIRE FABRIC - MINIMUM YIELD STRENGTH 70 KSI	DURING THE PRODUCTI
C. MATERIALS - CONCRETE THE CONCRETE FOR THE WALL SECTIONS SHALL BE COMPOSED OF PORTLAND CEMENT, FINE AND COARSE AGGREGATES, ADMIXTURES AND WATER. PORTLAND CEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM SPECIFICATION C150, TYPE I, II,	MANUFACTURER WILL F ACCORDANCE WITH AST SAMPLE, CONSISTING O RANDOMLY SELECTED F
OR III. THE AIR ENTRAINING ADMIXTURE SHALL CONTAIN 6% ± 2% ENTRAINED AIR, AND SLUMP SHALL BE MAINTAINED WITH THE RANGE OF 1" TO 4". THE SLUMP MAY BE INCREASED TO 7" PROVIDED THE INCREASE IS ACHIEVED BY THE ADDITION OF A	<i>CYLINDERS FOR COMPR 1'-0" SPECIMENS PREPA EVERY COMPRESSIBLE S CYLINDERS WILL BE CUR</i>
CHEMICAL WATER-REDUCING ADMIXTURE APPROVED BY THE ENGINEER.	AND TESTED AT APPROX COMPRESSIVE STRENGT ACCORDANCE WITH AST
D. MATERIALS AND REINFORCING HARDWARE REINFORCEMENT SHALL CONSIST OF WELDED WIRE FABRIC CONFORMING TO ASTM A185, OR DEFORMED BILLET-STEEL BARS CONFORMING TO ASTM A615, A616, OR A617, GRADE 60.	WILL DETERMINE THE IN ADDITION, 2 CYLINDERS ASTM C31 AND TESTED STRENGTH OF THESE TV
E. SHOP DRAWING REQUIREMENTS THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO MANUFACTURE. THE SHOP	ACCORDANCE WITH AST STRENGTH TEST RESULT STRENGTH OF THE PROL IF THE INITIAL STRENGT
<i>DRAWINGS SHALL INCLUDE THE FOLLOWING. - ALL STRUCTURAL DESIGN AND LOADING INFORMATION - A PLAN VIEW - ALL ELEVATION VIEWS - ALL DIMENSIONS</i>	STRENGTH IN EXCESS OF UTILIZED AS THE COMP PRODUCTION LOT AND WILL BE WAIVED FOR TH
MANUFACTURING SHALL NOT BEGIN UNTIL WRITTEN ACCEPTANCE OF THE SUBMITTED SHOP DRAWINGS HAS BEEN RECEIVED.	ACCEPTANCE OF A PROL COMPRESSIVE STRENGT TO 4 KSI. IF THE RESULT PRODUCTION LOT WILL
F. TESTING AND INSPECTION ACCEPTABILITY OF THE CONCRETE FOR THE PRECAST PANELS WILL BE DETERMINED ON THE BASIS OF COMPRESSION TESTS, CERTIFICATIONS AND VISUAL INSPECTIONS. THE CONCRETE STRENGTH REQUIREMENTS FOR THE PRECAST PANELS SHALL BE CONSIDERED ATTAINED REGARDLESS OF CURING AGE WHEN COMPRESSION TEST RESULTS INDICATE STRENGTH WILL	<i>THREE ACCEPTANCE CRI - 90% OF THE CON OVERALL PRODU - THE AVERAGE OF STRENGTH TEST - NO INDIVIDUAL FALL BELOW 3.6</i>
CONFORM TO 28-DAY SPECIFICATIONS AS STATED BELOW. THE MANUFACTURER SHALL FURNISH FACILITIES AND PERFORM ALL NECESSARY SAMPLING AND TESTING IN AN EXPEDITIOUS AND SATISFACTORY MANNER. PANELS UTILIZING TYPE I OR TYPE II CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL WHEN 7-DAY INITIAL STRENGTHS EXCEED 85 PERCENT OF THE 28-DAY REQUIREMENTS. PANELS UTILIZING TYPE III	IN THE EVENT THAT A PI SPECIFIED COMPRESSIV PRODUCTION LOT SHAL PREVAIL UNLESS THE M OBTAINS AND SUBMITS ENGINEER THAT THE STI
CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL PRIOR TO 28 DAYS ONLY WHEN COMPRESSIVE STRENGTH TEST RESULTS INDICATE THAT THE STRENGTH EXCEEDS THE 28-DAY SPECIFICATION.	PLACED WITHIN THE PA ACCEPTABLE. IF SUCH EN TAKEN FROM THE PANE SHALL BE OBTAINED AN SPECIFICATIONS OF AST
G. MANUFACTURE THE AGGREGATES, CEMENT AND WATER SHALL BE PROPORTIONED AND MIXED IN A BATCH MIXER TO PRODUCE A HOMOGENEOUS CONCRETE MEETING THE STRENGTH REQUIREMENTS OF THESE NOTES. THE PROPORTION OF PORTLAND CEMENT IN THE MIXTURE SHALL NOT BE LESS THAN 564 POUNDS PER CUBIC YARD OF CONCRETE.	
THE WALL SECTIONS SHALL BE CURED FOR A SUFFICIENT LENGTH OF TIME SO THAT THE CONCRETE WILL DEVELOP THE SPECIFIED COMPRESSIVE STRENGTH IN 28 DAYS OR LESS. ANY ONE OF THE METHODS OF CURING OR COMBINATION THEREOF SHALL BE USED:	
STEAM CURING - THE SECTIONS MAY BE LOW PRESSURE, STEAM CURED BY A SYSTEM THAT WILL MAINTAIN A MOIST ATMOSPHERE.	
WATER CURING - THE SECTIONS MAY BE WATER CURED BY ANY METHOD THAT WILL KEEP THE SECTIONS MOIST.	

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SED IN MANUFACTURE SHALL BE SUFFICIENTLY CURATE TO MAINTAIN THE SECTIONS DIMENSIONS ERMISSIBLE VARIATIONS GIVEN THESE NOTES. ALL FACE SHALL BE OF SMOOTH MATERIAL.

TION SHALL BE STORED IN SUCH A MANNER TO CKING OR DAMAGES.

CE OF THE REINFORCED CONCRETE PANELS SHALL THETIC FINISH AS SHOWN IN THE PLANS. CAULKING CAST PANELS SHALL BE IN ACCORDANCE WITH THE THE BACK SIDE OF THE REINFORCED CONCRETE HAVE A UNIFORM SURFACE FINISH AND SHALL BE DED TO ELIMINATE OPEN POCKETS OF AGGREGATE DISTORTIONS IN EXCESS OF 1/4".

ALL BE MANUFACTURED WITH ALL PANEL WITHIN 1/4".

OF THE CONCRETE PANELS WITH RESPECT TO STRENGTH WILL BE DETERMINED ON THE BASIS OF LOTS. A PRODUCTION LOT IS DEFINED AS A GROUP OF WILL BE REPRESENTED BY A SINGLE COMPRESSIVE MPLE AND WILL CONSIST OF EITHER 6 PANELS OR A PRODUCTION, WHICHEVER IS LESS.

PRODUCTION OF THE CONCRETE PANELS THE RER WILL RANDOMLY SAMPLE THE CONCRETE IN WITH ASTM C172. A SINGLE COMPRESSIVE STRENGTH SISTING OF A MINIMUM OF FOUR CYLINDERS. WILL BE ELECTED FOR EVERY PRODUCTION LOT.

R COMPRESSIVE STRENGTH TESTS SHALL BE 6" DIA. X NS PREPARED IN ACCORDANCE WITH ASTM C31. FOR RESSIBLE STRENGTH SAMPLE, A MINIMUM OF 2 ILL BE CURED IN THE SAME MANNER AS THE PANELS T APPROXIMATELY 7 DAYS. THE AVERAGE STRENGTH OF THESE CYLINDERS WHEN TESTED IN WITH ASTM C39, WILL PROVIDE A TEST RESULT WHICH INE THE INITIAL STRENGTH OF THE CONCRETE. IN CYLINDERS SHALL BE CURED IN ACCORDANCE WITH D TESTED AT 28 DAYS. THE AVERAGE COMPRESSIVE THESE TWO CYLINDERS, WHEN TESTED IN WITH ASTM C39, WILL PROVIDE A COMPRESSIVE ST RESULT WHICH WILL DETERMINE THE COMPRESSIVE THE PRODUCTION LOT.

STRENGTH TESTS RESULTS INDICATE A COMPRESSIVE EXCESS OF 4 KSI, THEN THESE TESTS RESULTS WILL BE HE COMPRESSIVE STRENGTH TEST RESULT FOR THE LOT AND THE REQUIREMENT FOR TESTING AT 28 DAYS ED FOR THAT PARTICULAR PRODUCTION LOT.

OF A PRODUCTION LOT WILL BE MADE IF THE STRENGTH TEST RESULT IS GREATER THAN OR EQUAL *IE RESULT IS LESS THAN 4 KSI, THE ACCEPTANCE OF THE* LOT WILL BE BASED ON ITS MEETING THE FOLLOWING TANCE CRITERIA:

F THE COMPRESSIVE STRENGTH TEST RESULTS FOR THE ALL PRODUCTION SHALL EXCEED 4 KSI. /ERAGE OF ANY SIX CONSECUTIVE COMPRESSIVE

GTH TEST RESULTS SHALL EXCEED 4 KSI.

DIVIDUAL COMPRESSIVE STRENGTH TEST RESULT SHALL ELOW 3.6 KSI.

THAT A PRODUCTION LOT FAILS TO MEET THE MPRESSIVE STRENGTH REQUIREMENTS. THE LOT SHALL BE REJECTED. SUCH REJECTION SHALL SS THE MANUFACTURER, AT THEIR OWN EXPENSE, SUBMITS EVIDENCE ACCEPTABLE TO THE AT THE STRENGTH AND QUALITY OF THE CONCRETE IN THE PANELS OF THE PRODUCTION LOT IS IF SUCH EVIDENCE CONSISTS OF TESTS MADE ON CORES THE PANELS WITHIN THE PRODUCTION LOT. THE CORES AINED AND TESTED IN ACCORDANCE WITH THE VS OF ASTM C42.

I. REJECTION PANELS SHALL BE SUBJECT TO REJECTION BECAUSE OF FAILURE TO MEET ANY OF THE REQUIREMENTS SPECIFIED ABOVE. IN ADDITION, ANY OR ALL OF THE FOLLOWING DEFECTS MAY BE SUFFICIENT CAUSE FOR REJECTION:

- DEFECTS THAT INDICATE IMPERFECT MOLDING - DEFECTS INDICATING HONEYCOMBED OR OPEN TEXTURED CONCRETE
- DEFECTS IN THE PHYSICAL CHARACTERISTICS OF THE CONCRETE, OR DAMAGE TO THE SEALING OF CONCRETE SURFACE TREATMENT OR TO AESTHETIC SURFACE
- TREATMENTS - CONCRETE CHIPS OR SPALLS THAT ARE LARGER THAN 4 INCHES WIDE OR 2 INCHES DEEP. REPAIR ALL CHIPS AND SPALLS THAT ARE SMALLER
- STAINED FORM FACES, DUE TO FORM OIL, CURING, OR OTHER CONTAMINANTS
- SIGNS OF AGGREGATE SEGREGATION
- CRACKS WIDER THAN 0.01 INCHES, PENETRATING MORE THAN 1 INCH OR LONGER THAN 20 PERCENT OF THE LENGTH OF THE FACE CONTAINING THE CRACK. REPAIR ALL CRACKS THAT ARE SMALLER
- PANELS THAT DO NOT MEET THE SPECIFIED DIMENSIONAL TOLERANCES
- UNUSABLE LIFTING INSERTS
- EXPOSED REINFORCING STEEL - INSUFFICIENT CONCRETE COMPRESSIVE STRENGTH

EITHER REPLACE DAMAGED PRECAST WALL PANELS OR DOCUMENT THE DAMAGE AND PROPOSE TO THE ENGINEER A REPAIR METHOD FOR THE DAMAGED PANEL: PERFORM REPAIRS WITH THE ACCEPTANCE OF THE ENGINEER. PROVIDE ACCEPTABLE REPLACEMENT PANELS FOR ANY THAT ARE REJECTED.

J. MARKING

THE DATE OF MANUFACTURE, THE PRODUCTION LOT NUMBER AND THE PIECE MARK SHALL BE CLEARLY SCRIBED ON THE BACK SURFACE OF EACH PANEL

K. CONCRETE LEVELING PAD

THE CONCRETE LEVELING PAD (MUD SLAB) SHALL BE CONSTRUCTED WITH CONCRETE HAVING A STRENGTH THAT IS NOT LESS THAN 3.5 KSI AND SHALL HAVE SUFFICIENT STRENGTH TO ADEQUATELY SUPPORT THE PANELS AT THE BOTTOM OF THE WALLIN A LEVEL POSITION DURING INSTALLATION.

A 4" (MIN.) THICK UNREINFORCED CONCRETE LEVELING PAD SHALL BE PROVIDED AS SHOWN ON THE PLANS. THE PAD SHALL BE CURED A MINIMUM OF 24 HOURS BEFORE PLACING WALL PANELS ON THE LEVELING PAD.

L. WALL ERECTION

PANELS ARE HANDLED BY MEANS OF A LIFTING DEVICE CONNECTED TO THE LIFTING INSERT WHICH IS CAST INTO THE UPPER EDGE OR BACK SIDE OF THE PANELS. ALL PANELS SHALL BE BRACED TO RESIST THE TEMPORARY CONSTRUCTION LOADS INCLUDING WIND LOADS, PRIOR TO FOOTING CONSTRUCTION.

M. BASIS OF PAYMENT

PAYMENT FOR ITEM 530- SPECIAL - STRUCTURES: PRECAST WALL PANELS COVERS ALL WORK DESCRIBED ABOVE.

VOLUME OF THE EMBEDDED PORTION OF PRECAST WALL HAS NOT BEEN SUBTRACTED FROM FOOTING CONCRETE VOLUME. QUANTITY TO BE ADJUSTED BASED ON SELECTED WALL FABRICATOR.

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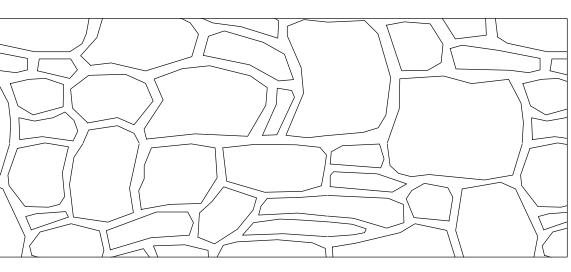
<u> // SPECIAL - 530 - STR</u> NCRETE FORMLINER)	UCTURES: AESTHETIC	<u>TREATMENT</u>	
ISTRUCTED USING ON ARCHITECTURAL SUR ROVED COMPANY ME THIS PROJECT, THE C TERN/COMPANY WAS	THETICS ON THE REFER NE OF THE PATTERNS DI RFACE ELEVATION AND EETING THE DETAILS SH CONTRACTOR WILL RESI S USED AND, AGAIN, M T AS DESCRIBED BELOV	TABLE FROM AN IOWN ON THIS PAGE. EARCH WHICH ATCH IT TO	
	TS REFERENCED BELOV ND APPEARANCE REFE		
VIDED FOR APPROVA PANEL DOES NOT M OGE SECTION, THE RE POSED PANEL SURFA VIDED REPEATEDLY, A CONTRACTOR SHALL HOWN IN THE PLAN, MLINING APPLICATIO SAME ARCHITECTUR ISED ON THE PROJEC IENT AND AGGREGAT	L BY THE DISTRICT 5 BE EET THE APPROVAL OF SULT WILL BE GROUND CE CHOSEN. THE TEST F AS NECESSARY, UNTIL A PROVIDE AN END SECT SHOWING THAT THEY ON AS DETAILED. THE M	THE DISTRICT 5 S TO REJECT THE PANEL WILL BE PPROVAL IS GRANTED. TON OF THE PARAPET, CAN ACHIEVE THE OCK-UP SHALL HAVE PATTERN INTENDED TO OF THE SAME BE USED TO E CONCRETE TEST	NOTES C0020-04.722 NDIANA & OHIO RAILWAY
ELS, AND ALL OTHER RK SHALL BE INCLUDE	NT INCLUDING THE SUI MATERIALS REQUIRED ED WITH THE ITEMIZED ES: AESTHETIC TREATME	TO COMPLETE THIS PAYMENT FOR ITEM	GENERAL NOTES E NO. FAI-CO020-0. AD OVER INDIANA
			GEN BRIDGE NO ICKERINGTON ROAD C
<u>ARCHITEC</u> FOLLOWING SHALL B	E <mark>TURAL SURFACE - ELEN</mark> BE USED:	<u>/ATION</u>	PICKE
COMPANY NAME:	PANEL SURFACE TREATMENT:	SPECIFICATIONS:	
C FORMLINERS, INC.	WASHINGTON DRYSTACK #1581	MAX RELIEF: 1½" LINER THICKNESS: 2⅛" STONE SIZE: 4" TO 24"	
CUSTOM ROCK INTERNATIONAL	NEW ENGLAND DRYSTACK #12003	MAX RELIEF: 1 ³ / ₈ " LINER THICKNESS: 2 ¹ / ₄ " STONE SIZE: 3" TO 24"	
APPROVED EQUAL	APPROVED EQUAL	APPROVED EQUAL	
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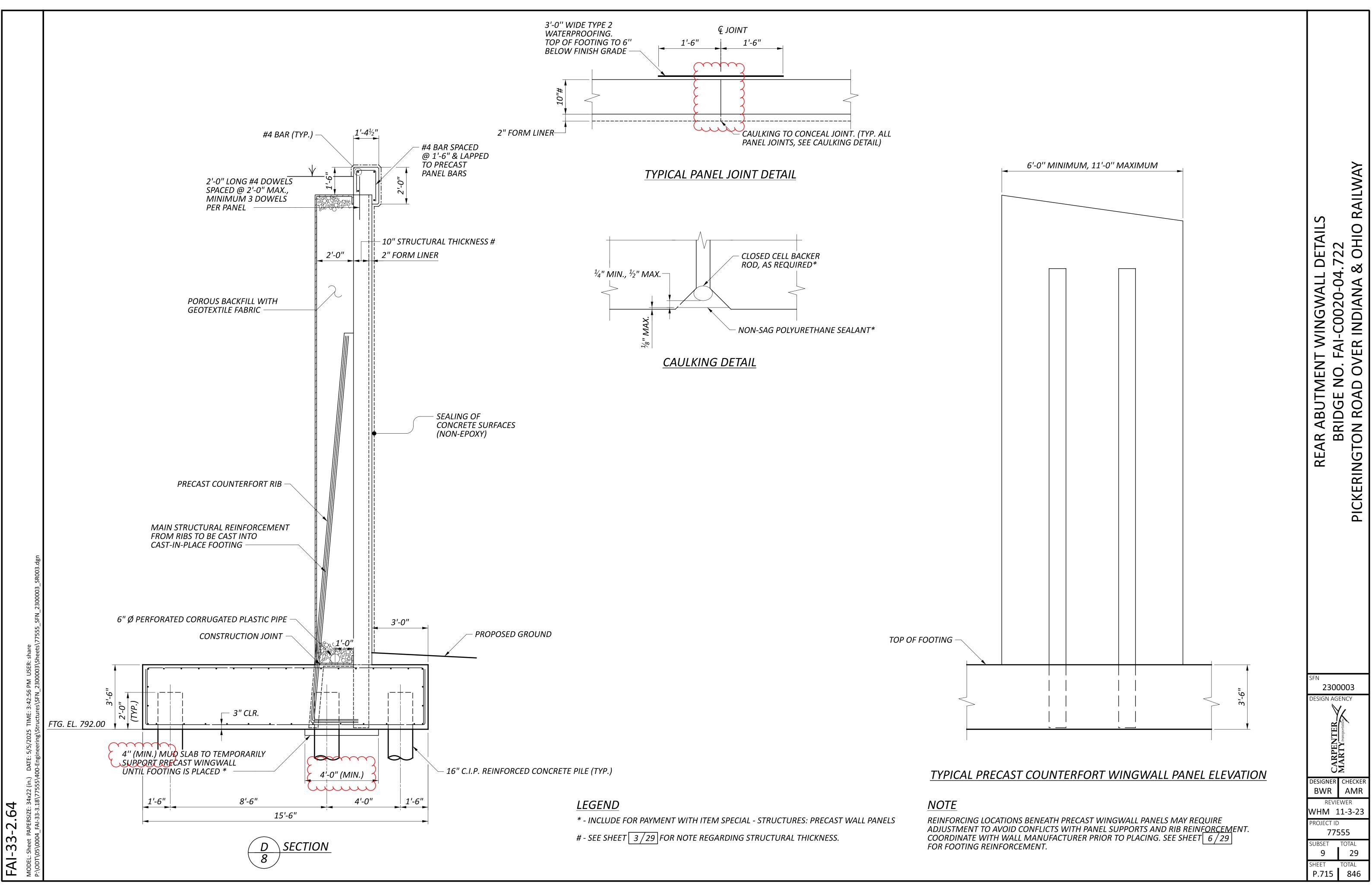
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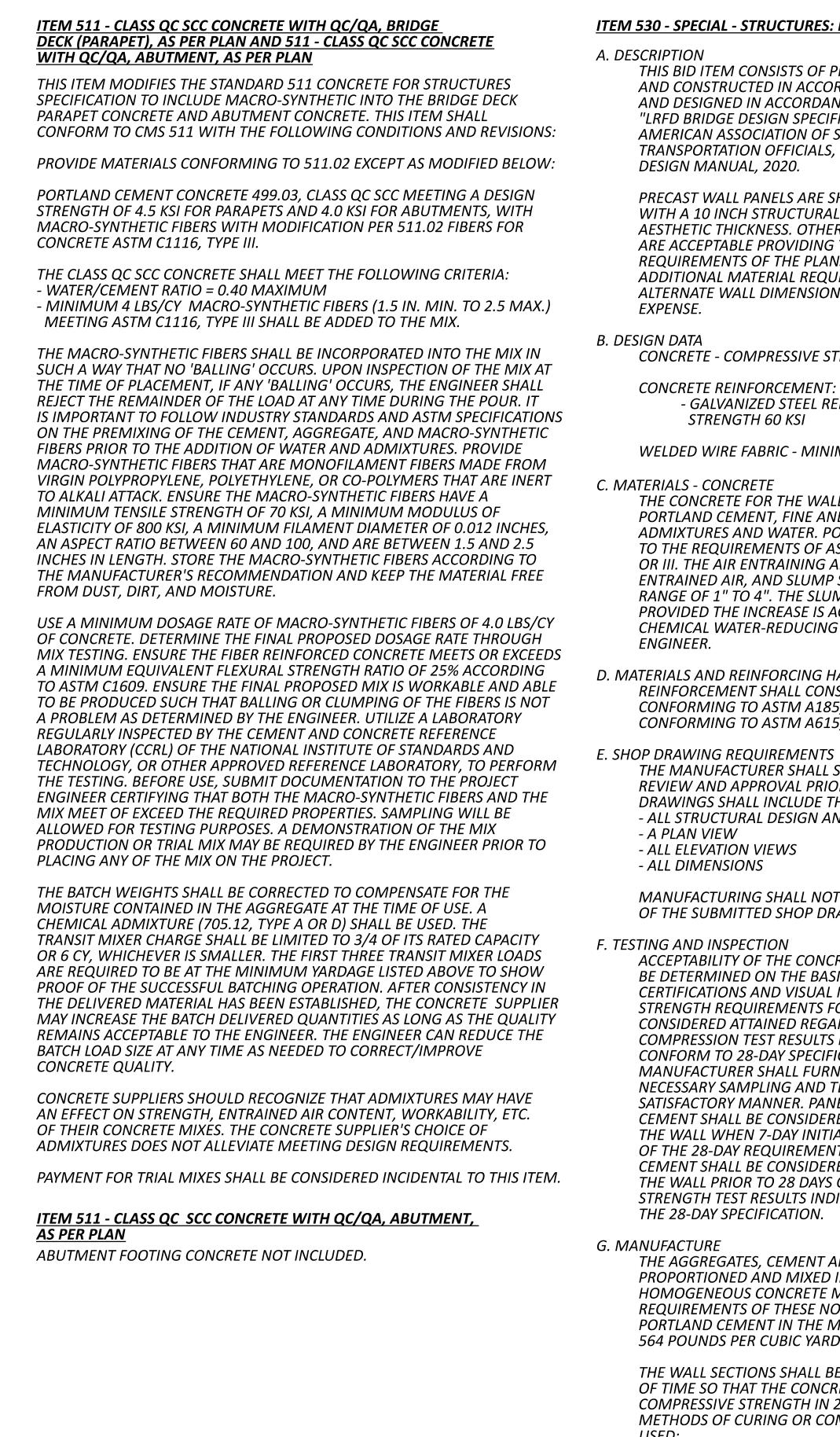
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STONE SIZE: 4" TO 24"			
EC FORMLINERS, INC. DRYSTACK #1581 LINER THICKNESS: 2 ⁵ / ₈ ' STONE SIZE: 4" TO 24"	COMPANY NAME:		SPECIFICATIONS:
	EC FORMLINERS, INC.		MAX RELIEF: 1½" LINER THICKNESS: 2⅓" STONE SIZE: 4" TO 24"
INTERNATIONAL DRVSTACK #12002 LINER THICKNESS: 21/4'			MAX RELIEF: 1 ³ / ₈ " LINER THICKNESS: 2 ¹ / ₄ " STONE SIZE: 3" TO 24"
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- ENGINEER.
- D. MATERIALS AND REINFORCING HARDWARE
- E. SHOP DRAWING REQUIREMENTS DRAWINGS SHALL INCLUDE THE FOLLOWING. - A PLAN VIEW
 - ALL ELEVATION VIEWS - ALL DIMENSIONS

MANUFACTURING SHALL NOT BEGIN UNTIL WRITTEN APPROVAL OF THE SUBMITTED SHOP DRAWINGS HAS BEEN RECEIVED.

- F. TESTING AND INSPECTION THE 28-DAY SPECIFICATION.
- G. MANUFACTURE

THE AGGREGATES, CEMENT AND WATER SHALL BE PROPORTIONED AND MIXED IN A BATCH MIXER TO PRODUCE A HOMOGENEOUS CONCRETE MEETING THE STRENGTH REQUIREMENTS OF THESE NOTES. THE PROPORTION OF PORTLAND CEMENT IN THE MIXTURE SHALL NOT BE LESS THAN 564 POUNDS PER CUBIC YARD OF CONCRETE.

THE WALL SECTIONS SHALL BE CURED FOR A SUFFICIENT LENGTH OF TIME SO THAT THE CONCRETE WILL DEVELOP THE SPECIFIED COMPRESSIVE STRENGTH IN 28 DAYS OR LESS. ANY ONE OF THE METHODS OF CURING OR COMBINATION THEREOF SHALL BE USED:

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ITEM 530 - SPECIAL - STRUCTURES: PRECAST WALL PANELS

THIS BID ITEM CONSISTS OF PRECAST WALLS MANUFACTURED AND CONSTRUCTED IN ACCORDANCE WITH THIS SPECIFICATION AND DESIGNED IN ACCORDANCE WITH THE 9th EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2020 AND THE ODOT BRIDGE

PRECAST WALL PANELS ARE SHOWN AND DETAILED IN THE PLANS WITH A 10 INCH STRUCTURAL THICKNESS AND A 2 INCH AESTHETIC THICKNESS. OTHER STRUCTURAL WALL THICKNESSES ARE ACCEPTABLE PROVIDING THEY MEET ALL THE REQUIREMENTS OF THE PLANS AND SPECIFICATIONS. ANY ADDITIONAL MATERIAL REQUIRED TO ACCOMMODATE ALTERNATE WALL DIMENSIONS SHALL BE AT THE CONTRACTOR'S

CONCRETE - COMPRESSIVE STRESS 4.0 KSI

- GALVANIZED STEEL REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI

WELDED WIRE FABRIC - MINIMUM YIELD STRENGTH 70 KSI

THE CONCRETE FOR THE WALL SECTIONS SHALL BE COMPOSED OF PORTLAND CEMENT, FINE AND COARSE AGGREGATES, ADMIXTURES AND WATER. PORTLAND CEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM SPECIFICATION C150, TYPE I, II, OR III. THE AIR ENTRAINING ADMIXTURE SHALL CONTAIN 6% ± 2% ENTRAINED AIR, AND SLUMP SHALL BE MAINTAINED WITH THE RANGE OF 1" TO 4". THE SLUMP MAY BE INCREASED TO 7" PROVIDED THE INCREASE IS ACHIEVED BY THE ADDITION OF A CHEMICAL WATER-REDUCING ADMIXTURE APPROVED BY THE

REINFORCEMENT SHALL CONSIST OF WELDED WIRE FABRIC CONFORMING TO ASTM A185, OR DEFORMED BILLET-STEEL BARS CONFORMING TO ASTM A615, A616, OR A617, GRADE 60.

THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO MANUFACTURE. THE SHOP - ALL STRUCTURAL DESIGN AND LOADING INFORMATION

ACCEPTABILITY OF THE CONCRETE FOR THE PRECAST PANELS WILL BE DETERMINED ON THE BASIS OF COMPRESSION TESTS, CERTIFICATIONS AND VISUAL INSPECTIONS. THE CONCRETE STRENGTH REOUIREMENTS FOR THE PRECAST PANELS SHALL BE CONSIDERED ATTAINED REGARDLESS OF CURING AGE WHEN COMPRESSION TEST RESULTS INDICATE STRENGTH WILL CONFORM TO 28-DAY SPECIFICATIONS AS STATED BELOW. THE MANUFACTURER SHALL FURNISH FACILITIES AND PERFORM ALL NECESSARY SAMPLING AND TESTING IN AN EXPEDITIOUS AND SATISFACTORY MANNER. PANELS UTILIZING TYPE I OR TYPE II CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL WHEN 7-DAY INITIAL STRENGTHS EXCEED 85 PERCENT OF THE 28-DAY REQUIREMENTS. PANELS UTILIZING TYPE III CEMENT SHALL BE CONSIDERED ACCEPTABLE FOR PLACEMENT IN THE WALL PRIOR TO 28 DAYS ONLY WHEN COMPRESSIVE STRENGTH TEST RESULTS INDICATE THAT THE STRENGTH EXCEEDS

STEAM CURING - THE SECTIONS MAY BE LOW PRESSURE, STEAM CURED BY A SYSTEM THAT WILL MAINTAIN A MOIST

WATER CURING - THE SECTIONS MAY BE WATER CURED BY ANY METHOD THAT WILL KEEP THE SECTIONS MOIST.

THE FORMS USED IN MANUFACTURE SHALL BE SUFFICIENTLY RIGID AND ACCURATE TO MAINTAIN THE SECTIONS DIMENSIONS WITHIN THE PERMISSIBLE VARIATIONS GIVEN THESE NOTES. ALL CASTING SURFACE SHALL BE OF SMOOTH MATERIAL.

THE WALL SECTION SHALL BE STORED IN SUCH A MANNER TO PREVENT CRACKING OR DAMAGES.

THE FRONT FACE OF THE REINFORCED CONCRETE PANELS SHALL HAVE AN AESTHETIC FINISH AS SHOWN IN THE PLANS. CAULKING BETWEEN PRECAST PANELS SHALL BE IN ACCORDANCE WITH THE PLAN DETAILS. THE BACK SIDE OF THE REINFORCED CONCRETE PANELS SHALL HAVE A UNIFORM SURFACE FINISH AND SHALL BE ROUGH SCREEDED TO ELIMINATE OPEN POCKETS OF AGGREGATE AND SURFACE DISTORTIONS IN EXCESS OF 1/4".

ALL PANELS SHALL BE MANUFACTURED WITH ALL PANEL DIMENSIONS WITHIN 1/4".

H. COMPRESSIVE STRENGTH

ACCEPTANCE OF THE CONCRETE PANELS WITH RESPECT TO COMPRESSIVE STRENGTH WILL BE DETERMINED ON THE BASIS OF PRODUCTION LOTS. A PRODUCTION LOT IS DEFINED AS A GROUP OF PANELS THAT WILL BE REPRESENTED BY A SINGLE COMPRESSIVE STRENGTH SAMPLE AND WILL CONSIST OF EITHER 6 PANELS OR A SINGLE DAY'S PRODUCTION, WHICHEVER IS LESS.

DURING THE PRODUCTION OF THE CONCRETE PANELS THE MANUFACTURER WILL RANDOMLY SAMPLE THE CONCRETE IN ACCORDANCE WITH ASTM C172. A SINGLE COMPRESSIVE STRENGTH SAMPLE. CONSISTING OF A MINIMUM OF FOUR CYLINDERS. WILL BE RANDOMLY SELECTED FOR EVERY PRODUCTION LOT.

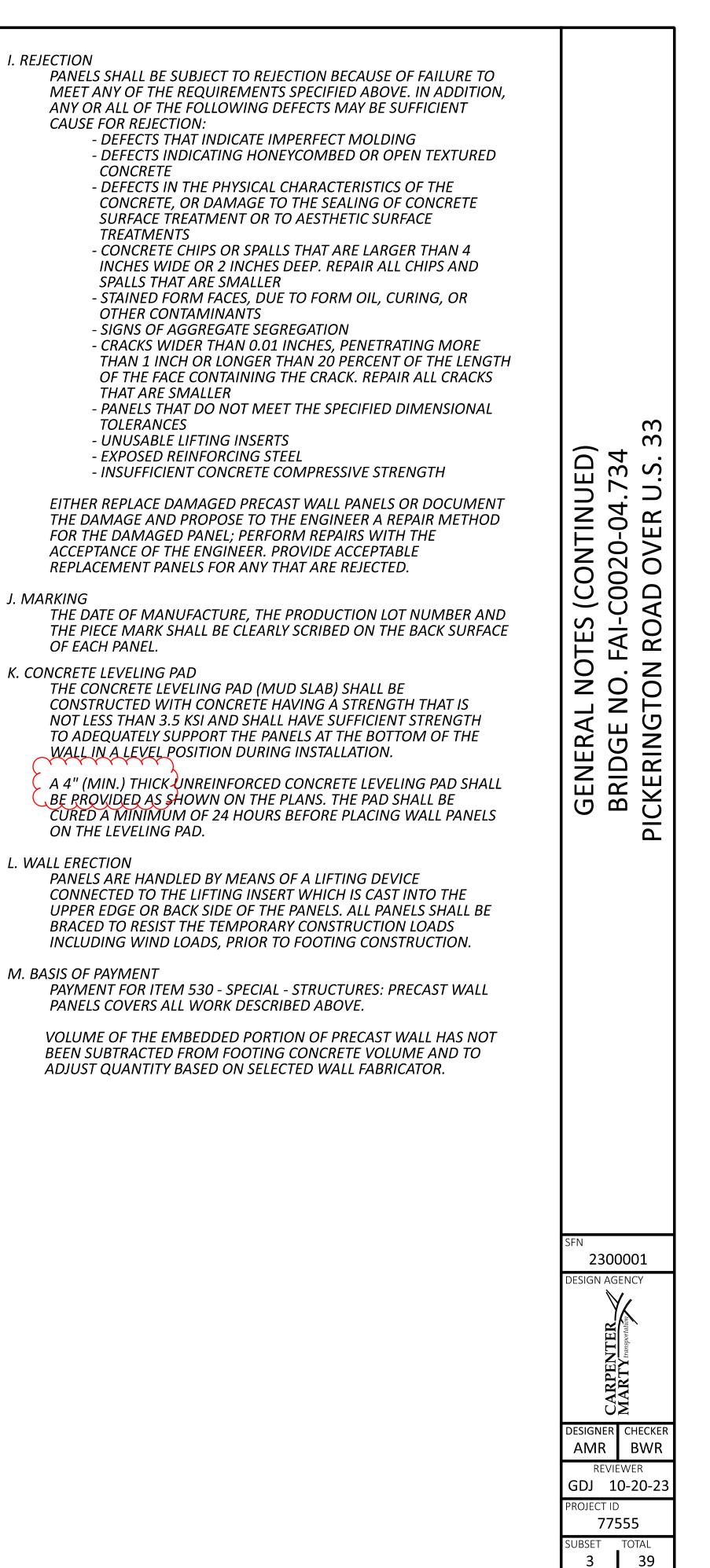
CYLINDERS FOR COMPRESSIVE STRENGTH TESTS SHALL BE 6" DIA. X 1'-0" SPECIMENS PREPARED IN ACCORDANCE WITH ASTM C31. FOR EVERY COMPRESSIBLE STRENGTH SAMPLE, A MINIMUM OF 2 CYLINDERS WILL BE CURED IN THE SAME MANNER AS THE PANELS AND TESTED AT APPROXIMATELY 7 DAYS. THE AVERAGE COMPRESSIVE STRENGTH OF THESE CYLINDERS WHEN TESTED IN ACCORDANCE WITH ASTM C39, WILL PROVIDE A TEST RESULT WHICH WILL DETERMINE THE INITIAL STRENGTH OF THE CONCRETE. IN ADDITION, 2 CYLINDERS SHALL BE CURED IN ACCORDANCE WITH ASTM C31 AND TESTED AT 28 DAYS. THE AVERAGE COMPRESSIVE STRENGTH OF THESE TWO CYLINDERS, WHEN TESTED IN ACCORDANCE WITH ASTM C39, WILL PROVIDE A COMPRESSIVE STRENGTH TEST RESULT WHICH WILL DETERMINE THE COMPRESSIVE STRENGTH OF THE PRODUCTION LOT.

IF THE INITIAL STRENGTH TESTS RESULTS INDICATE A COMPRESSIVE STRENGTH IN EXCESS OF 4 KSI, THEN THESE TESTS RESULTS WILL BE UTILIZED AS THE COMPRESSIVE STRENGTH TEST RESULT FOR THE **PRODUCTION LOT AND THE REQUIREMENT FOR TESTING AT 28 DAYS** WILL BE WAIVED FOR THAT PARTICULAR PRODUCTION LOT.

ACCEPTANCE OF A PRODUCTION LOT WILL BE MADE IF THE COMPRESSIVE STRENGTH TEST RESULT IS GREATER THAN OR EQUAL TO 4 KSI. IF THE RESULT IS LESS THAN 4 KSI, THE ACCEPTANCE OF THE PRODUCTION LOT WILL BE BASED ON ITS MEETING THE FOLLOWING THREE ACCEPTANCE CRITERIA:

- 90% OF THE COMPRESSIVE STRENGTH TEST RESULTS FOR THE OVERALL PRODUCTION SHALL EXCEED 4 KSI.
- THE AVERAGE OF ANY SIX CONSECUTIVE COMPRESSIVE
- STRENGTH TEST RESULTS SHALL EXCEED 4 KSI.
- NO INDIVIDUAL COMPRESSIVE STRENGTH TEST RESULT SHALL FALL BELOW 3.6 KSI.

IN THE EVENT THAT A PRODUCTION LOT FAILS TO MEET THE SPECIFIED COMPRESSIVE STRENGTH REQUIREMENTS, THE PRODUCTION LOT SHALL BE REJECTED. SUCH REJECTION SHALL PREVAIL UNLESS THE MANUFACTURER. AT THEIR OWN EXPENSE. OBTAINS AND SUBMITS EVIDENCE ACCEPTABLE TO THE ENGINEER THAT THE STRENGTH AND QUALITY OF THE CONCRETE PLACED WITHIN THE PANELS OF THE PRODUCTION LOT IS ACCEPTABLE. IF SUCH EVIDENCE CONSISTS OF TESTS MADE ON CORES TAKEN FROM THE PANELS WITHIN THE PRODUCTION LOT. THE CORES SHALL BE OBTAINED AND TESTED IN ACCORDANCE WITH THE SPECIFICATIONS OF ASTM C42.



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