
**FINAL REPORT
SUBGRADE EXPLORATION REPORT
FRA-71/270-28.27/25.99A
SAFETY AND SYSTEM PRESERVATION
FRANKLIN COUNTY, OHIO
PID#: 105435**

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NEAS PROJECT 21-0012

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EXECUTIVE SUMMARY

The Ohio Department of Transportation (ODOT) has proposed an interchange improvement project (FRA-71/270-28.27/25.99, PID# 105435) for the Interstate Route (IR) 270 and IR-71 on the north side of Columbus, Franklin County, Ohio. It is our understanding that the overall project objective is to improve capacity to the IR-270 and IR-71 interchange. The interchange and mainline improvements purposed to accomplish this objective include: 1) widening of the IR-71 freeway segment within the project limits; 2) the construction/reconstruction of 5 connecting ramps (Ramp K, M, N, O, P); 3) the replacement of the existing bridge structure FRA-00071-28.265 carrying Ramp K (IR-270 WB to IR-71 SB) over IR-71; 4) the replacement of the existing bridge structure FRA-00071-28.294 carrying Ramp O (IR-71 NB to IR-270 WB) over IR-71; and, 5) the superstructure replacement of the existing bridge structure FRA-00270-25.990A carrying Ramp K (IR-270 WB to IR-71 SB) over Ramp O.

National Engineering & Architectural Services, Inc. (NEAS) has been contracted to perform geotechnical engineering services for the project. The purpose of the geotechnical engineering services was to perform geotechnical explorations within the project limits to obtain information concerning the subsurface soil and groundwater conditions relevant to the design and construction of the project. NEAS performed the site reconnaissance and exploration program for Part 1 of the project between April 9, 2021 and April 20, 2021 and Part 2 of the project between July 26 and August 10, 2022. Part 1 involved the subgrade exploration for Ramp P and Ramp M. Part 2 involved the subsurface exploration for Ramp K, Ramp N, Ramp O and three bridges. The entire project included 43 borings drilled to depths between 7.5 ft and 120.0 ft below ground surface (bgs) and 7 pavement cores for subgrade characterization and bridge design purposes. Additionally, between October 23 and October 25, 2023, NEAS also conducted six project borings for two planned retaining walls, which were subsequently eliminated from the design.

The subgrade conditions in the project area are relatively consistent and are generally comprised of primarily cohesive natural overburden soils (A-4a, A-6a, A-6b, and A-7-6) and minor non-cohesive overburden soils (A-1-a, A-1-b, A-2-4). With respect to sulfate within the subgrade soil, based on the project laboratory testing program, three soil samples present a sulfate content value greater than 5,000 ppm within all of the project borings performed. Groundwater was not encountered during drilling and after drilling in all the project borings performed. Bedrock was not encountered in all the project borings within the subgrade depth.

High sulfate content soils were encountered at the project site. Three soil samples in Borings B-008-0-21, B-014-0-21 and B-016-0-21 present a sulfate content greater than 3,000 ppm, however, less than 5,000 ppm. Three soil samples in Borings B-009-0-21, B-010-0-21 and B-040-021 present a sulfate content greater than 5,000 ppm.

Based on our evaluation of the subsurface conditions and our geotechnical engineering analyses of the proposed interchange improvement project, it is our opinion that subgrade conditions are generally satisfactory, and pavement can be designed without the need for extreme levels of remediation. Unstable subgrade conditions, including areas of weak soils and high moisture content soils, were encountered throughout 8 percent of the proposed pavement widening area along IR-71 NB and the IR-71 and IR-270 interchange associated ramps. NEAS recommends a global 12" cement stabilization for all ramps and IR-71 NB from Begin to Station 156+67 and undercut and replacement for IR-71 NB from Station 156+67 to End. Excavations are estimated to extend to the depth of 12 inches, with the excavated material being replaced with material in accordance with Section F "Excavate and Replace (Item 204)" of the ODOT GB1. Stabilization limits should extend 18-inches beyond the edge of the proposed paved roadway, shoulder or median and it is recommended removing any topsoil, existing pavement materials or abandoned structure foundation materials.

Overall, NEAS's opinion is that the subgrade soils will provide adequate pavement support, assuming the pavement is designed and constructed in accordance with the recommendations provided within this report, as well as all applicable ODOT standards and specifications.

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Safety and System Preservation

Franklin County, Ohio

PID: 105435

1. INTRODUCTION

1.1. General

National Engineering & Architectural Services, Inc. (NEAS) presents our Subgrade Exploration Report for the proposed FRA-71/270-28.27/25.99A Safety and System Preservation project, Franklin County, Ohio. The Safety and System Preservation project involves: 1) widening of the IR-71 freeway segment within the project limits; 2) the construction/reconstruction of 5 connecting ramps (Ramp K, M, N, O, P); 3) the replacement of the existing bridge structure FRA-00071-28.265 carrying Ramp K (IR-270 WB to IR-71 SB) over IR-71; 4) the replacement of the existing bridge structure FRA-00071-28.294 carrying Ramp O (IR-71 NB to IR-270 WB) over IR-71; and, 5) the superstructure replacement of the existing bridge structure FRA-00270-25.990A carrying Ramp K (IR-270 WB to IR-71 SB) over Ramp O.

This report presents a summary of the project encountered surficial and subsurface conditions and our recommendations for subgrade stabilization and pavement design parameters for I-71 and the corresponding ramps. The analysis performed as part of this report has been performed in accordance with ODOT's January 2019 revision of *Geotechnical Bulletin 1* (GB1) (ODOT [1], 2021) and *Pavement Design Manual* (PDM) (ODOT, 2020).

The exploration was conducted in general accordance with NEAS's proposals to TranSystems, dated January 8, 2021 and February 25, 2022, and ODOT's July 2022 revision of *Specifications for Geotechnical Explorations* (SGE) (ODOT, 2022).

The scope of work performed by NEAS as part of the referenced project included: a review of published geotechnical information; performing 43 total test borings (all of which were utilized within this report as part of the subgrade exploration) and 7 pavement cores; laboratory testing of soil samples in accordance with the SGE; performing geotechnical engineering analysis to assess subgrade stabilization requirements and recommended pavement design parameters; and development of this summary report.

2. GEOLOGY AND OBSERVATIONS OF THE PROJECT

2.1. Geology and Physiography

The project site is located within the Columbus Lowland Till Plains, a subdivision of the Southern Ohio Loamy Till Plain. This is a moderately low relief (25 ft) lowland surrounded in all directions by relative uplands, having a broad regional slope toward the Scioto Valley, containing many larger streams. Elevations of the region range from 600 to 850 ft above mean sea level (amsl) (950 ft amsl near Powell Moraine). The geology within this region is described as Wisconsinan-age till that is high lime in the west to medium-lime in the east. The geology is also described as containing extensive outwash in Scioto Valley overlying deep Devonian- to Mississippian-age carbonate rocks, shales and siltstones (ODGS, 1998).

Based on the Bedrock Geologic Units Map of Ohio (USGS & ODGS, 2005), bedrock within the project limits is comprised of Devonian-age Ohio Shale. The Devonian-age Ohio Shale is about 359 to 385 million years old, and the sedimentary rocks mainly consist of shale and siltstone with some sandstone. The shale unit at the project site is brownish black to greenish gray, weathers brown, carbonaceous to clayey, laminated to thin bedded, and is fissile parting. This unit is carbonated and/or siderite concretions in lowermost 50 feet, with petroliferous odor and about 250 to 500+ feet thick. Based on the ODNR

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bedrock topography map of Ohio, bedrock elevations at the project site can be expected to range from 800 to 850 ft amsl, putting bedrock at a depth of 65 to 110 ft below ground surface (bgs).

The soils at the project site have been mapped (Web Soil Survey) by the Natural Resources Conservation Service as being gently rolling Udoorthents-Urban land complex (USDA, 2015). The units of the Udoorthents-Urban series account for 90% soils with none flood. The units can be classified as A-1-a, A-1-b, A-2-4, A-4a, A-6a, A-6b and A-7-6 soils according to the AASHTO method of soil classification.

2.2. Hydrology/Hydrogeology

Groundwater can be expected at an elevation consistent with that of the major local surface water bodies. A major regional hydraulic influence is the Alum Creek located about 2.0 miles to the east. The Water wells near the project site were noted to have a static water from 23 ft to 95 ft below ground surface.

Local variations in the groundwater table may exist in one of two reasons. First, if there has been extensive groundwater abstraction, water levels may be depressed by tens of feet over significantly large areas. Second, the presence of discontinuous bodies of glacial till provides the opportunity for localized pockets of perched groundwater to form.

The proposed project site is not located within a 0.2% and 1% Annual Chance Flood Hazard area based on available mapping by the Federal Emergency Management Agency's (FEMA) National Flood Hazard mapping program (FEMA, 2019).

2.3. Mining and Oil/Gas Production

No abandoned mines were noted on ODNR's Abandoned Underground Mine Locator immediately adjacent to the project's boundaries (ODNR [1], 2016).

No gas or oil wells were noted on ODNR's Ohio Oil & Gas Locator within the immediate vicinity of the project's boundaries (ODNR [2], 2016).

2.4. Historical Records and Previous Phases of Project Exploration

A historic record search was performed through ODOT's Transportation Information Management System (TIMS). Several historic projects were available for review within the limits of the FRA-71/270-28.27/25.99A project.

- FRA-270-24.47, Reconstruction and Widening, 2006
- FRA-270-15.50N, Project No. 011502, 1963
- FRA-270-16.65N, Project No. 011503, 1964

2.5. Field Reconnaissance

Field reconnaissance visits for the overall project area were conducted on April 9, 2021 and May 3, 2022 for Part 1 and Part 2, respectively, within the project limits. Site conditions, including the existing land conditions and pavement conditions, were noted, and photographed during the visit. Photographs of notable features and a summary of our observations by road segment are provided below.

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2.5.1. Land Use and Cover

The land use of most of the project area consists of ODOT ROW (Right of Way), commercial properties (i.e., single family homes, apartments, etc.) and woodland.

2.5.2. Ramp P and Ramp M

In general, the pavement condition along Ramp P and Ramp M was observed to be good with signs of surface wear. The travel lanes were noted to be in markedly better condition than the shoulders along this section of roadway. Moderate severity longitudinal and transverse cracking was observed along this section as well as crack sealing deficiencies (Photograph 1). The roadway in this section sits atop a small embankment running from level with IR-270 up to the level of the ramp bridge. The embankment slopes are roughly 3H:1V (3 horizontal to one vertical). The roadway is relatively level in this section. The roadway drains to drainage ditches at the bottom of each side of the embankment (Photograph 2). The area is lightly vegetated for the most part with some signs of standing water were observed in the drainage ditches such as heavy vegetation and cattails (Photograph 2). The area appeared to be stable with no signs of geotechnical instability.

Photograph 1: Overall Pavement Condition of Ramp P and Ramp M



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Photograph 2: Signs of Standing Water in Drainage Ditch



2.5.3. IR-71 NB From Interchange to Project End

In general, the pavement condition along this section of the project was observed to be good with signs of surface wear. The travel lanes were noted to be in markedly better condition than the shoulders along this section of roadway. Moderate severity longitudinal and transverse cracking was observed along this section as well as crack sealing deficiencies (Photograph 3). The roadway in this section sits in a cut with embankment slopes rising up on either side of the highway to the level of the surrounding land. The embankment slopes are roughly 2H:1V (2 horizontal to one vertical). The roadway is relatively level in this section. The roadway drains to drainage ditches at the bottom of each side of the embankment (Photograph 4). The area is lightly vegetated for the most part with some signs of standing water were observed in the drainage ditches. The area appeared to be stable with no signs of geotechnical instability.

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Photograph 3: Overall Pavement Condition of IR-71 NB



Photograph 4: Drainage Ditch on at Bottom of Embankment



2.5.4. Bridge FRA-00071-27.94

The existing bridge carrying Ramp P from IR-270 EB to IR-71 NB consists of a five-span, multi-beam bridge with stub type abutments and cap and column type piers (Photograph 5). In the area of the referenced bridge, the terrain is roughly level with the surrounding area which rises very gently from north to south. Signs of instability were not observed during our site visit. The overall bridge structure appeared to be in good condition with few signs of distress observed. The spill-through slopes appeared to be at roughly 2H:1V (2 horizontal to one vertical) slopes and in good condition and protected from

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erosion by rip rap. No apparent signs of distress due to geotechnical concerns were noted during our field reconnaissance visit.

The bridge deck and concrete wearing course was observed to be in good condition with minor severity pop-outs being common (Photograph 6). With respect to drainage, the bridge deck and adjacent pavement appeared to be well drained, with no signs of ponding or drainage issues observed during our field visit. The adjacent ramp appeared to drain to drainage ditches that runs parallel to the roadway. The bridge deck drained off of the north side of the bridge at either end of the bridge where the concrete guard rail terminates.

Photograph 5: Cap and Column Bridge Piers



Photograph 6: Bridge Deck Wearing Course



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2.5.5. *Ramp K*

In general, the pavement condition of Ramp K east of IR-71 was observed to be good with some signs of weathering and surface wear. Moderate severity edge cracking was common along this section of the ramp, as well as crack sealing deficiencies (Photograph 7). The pavement condition of Ramp K west of IR-71 was observed to be in markedly worse condition than that east of IR-71. Moderate severity longitudinal and transverse cracking was common along this section of the ramp, as well as map cracking, and patching (Photograph 8). The ramp starts out on embankments above the surrounding land with slopes of roughly 3H:1V (3 ft horizontal to 1 ft vertical) and then transitions to level with the surrounding land in the area west of IR-71 and North of IR-270. The roadway in this section of the project drains directly off the northern shoulder of the ramp east of IR-71. The ramp then drains off of both shoulders of the ramp between the two aforementioned bridges. Finally, the ramp drains off the southern shoulder of the ramp west of IR-71. The area is moderately vegetated, and signs of standing water such as cattails were observed in the drainage ditch off the southern shoulder of the ramp west of IR-71. No signs of issues due to geotechnical conditions were observed.

Photograph 7: Overall Pavement Condition of Ramp K East of IR-71



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Photograph 8: Overall Pavement Condition of Ramp K West of IR-71



2.5.6. *Ramp O*

In general, the pavement condition of Ramp O was observed to be good with some signs of weathering and surface wear. Moderate severity edge cracking was common along the ramp, as well as crack sealing deficiencies (Photograph 9). The ramp starts out level with the surrounding land near IR-71 N, then transitions to embankments above the surrounding land with slopes of roughly 3.5H:1V (3.5 ft horizontal to 1 ft vertical) right after it passes under ramp K. The roadway in this section of the project drains directly off the southwestern shoulder of the ramp. The area is moderately vegetated, and signs of standing water such as cattails were observed in the drainage ditch south of where Ramp O passes under Ramp K. No signs of issues due to geotechnical conditions were observed.

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Photograph 9: Overall Pavement Condition of Ramp O



3. GEOTECHNICAL EXPLORATION

3.1. Roadway Exploration Program

The subsurface exploration for the project was conducted by NEAS between April 9, 2021, and April 20, 2021, and between July 26 and August 10, 2021, and included 43 borings drilled to the depths between 7.5 ft and 120.0 ft bgs. The boring locations were selected by NEAS in general accordance with the guidelines contained in the SGE with the intent to evaluate subsurface soil and groundwater conditions. Borings were typically located either within existing pavement areas that are planned to undergo full-depth replacement or within areas where widening is planned. Target boring locations were located in the field by NEAS prior to drilling utilizing handheld GPS equipment and the boring locations were drilled in areas that were not restricted by underground utilities or dictated by terrain (i.e. steep embankment slopes). Each as-drilled project boring location and corresponding ground surface elevation was surveyed in the field following drilling. Each individual project boring log (included within Appendix B) includes the recorded boring latitude and longitude location (based on the surveyed Ohio State Plane South, NAD83, location) and the corresponding ground surface elevation, as summarized in Table 1.

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Table 1: Project Boring Summary

Boring Number	Latitude	Longitude	Elevation (NAVD 88) (ft)	Alignment	Station	Offset	Depth (ft)	Substructure
B-001-0-21	40.110037	-82.977651	913.0	I-71	130+48	62' RT.	7.5	Subgrade
B-002-0-21	40.111052	-82.976900	914.1	I-71	134+73	77' RT.	7.5	Subgrade
B-003-0-21	40.112010	-82.976279	913.7	I-71	138+60	75' RT.	7.5	Subgrade
B-004-0-21	40.112998	-82.975669	911.1	I-71	142+56	79' RT.	7.5	Subgrade
B-005-0-21	40.113975	-82.975211	908.2	I-71	146+32	58' RT.	7.5	Subgrade
B-006-0-21	40.115133	-82.974503	903.0	I-71	150+94	91' RT.	7.5	Subgrade
B-007-0-21	40.116108	-82.974071	899.4	I-71	154+66	89' RT.	7.5	Subgrade
B-008-0-21	40.117174	-82.973656	895.2	I-71	158+69	85' RT.	7.5	Subgrade
B-009-0-21	40.118246	-82.973248	891.0	I-71	162+72	91' RT.	7.5	Subgrade
B-010-0-21	40.119281	-82.972942	887.8	I-71	166+56	86' RT.	7.5	Subgrade
B-011-0-21	40.120386	-82.972642	885.9	I-71	170+65	86' RT.	7.5	Subgrade
B-012-0-21	40.121451	-82.972399	886.8	I-71	174+56	86' RT.	7.5	Subgrade
B-013-0-21	40.122537	-82.972200	889.2	I-71	178+53	86' RT.	7.5	Subgrade
B-014-0-21	40.123658	-82.972045	891.6	I-71	182+61	85' RT.	7.5	Subgrade
B-015-0-21	40.124754	-82.971942	894.1	I-71	186+58	84' RT.	7.5	Subgrade
B-016-0-21	40.125854	-82.971865	896.6	I-71	190+59	84' RT.	7.5	Subgrade
B-017-0-21	40.126945	-82.971739	898.8	I-71	194+58	96' RT.	7.5	Subgrade
B-018-0-21	40.110560	-82.989002	921.6	Ramp M	84+94	3' LT.	7.5	Subgrade
B-019-0-21	40.110619	-82.987640	920.4	Ramp M	88+73	46' LT.	7.5	Subgrade
B-020-0-21	40.110342	-82.986326	917.4	Ramp M	92+48	4' LT.	7.5	Subgrade
B-021-0-21	40.109798	-82.985064	918.6	Ramp M	96+55	5' RT.	7.5	Subgrade
B-022-0-21	40.109199	-82.984109	920.6	Ramp M	100+01	3' RT.	7.5	Subgrade
B-023-0-21	40.108675	-82.982948	923.6	Ramp M	103+72	62' LT.	7.5	Subgrade
B-024-0-21	40.107974	-82.981926	925.6	Ramp M	107+55	49' LT.	7.5	Subgrade
B-025-0-21	40.107701	-82.980661	935.5	Ramp P	1011+38	22' LT.	7.5	Subgrade
B-026-0-21	40.108377	-82.978429	928.1	Ramp P	1018+46	4' RT.	7.5	Subgrade
B-027-0-21	40.109351	-82.977947	913.7	Ramp P	1022+29	18' LT.	7.5	Subgrade
B-028-0-21	40.110243	-82.972635	921.3	Ramp K	13+63	23' RT.	7.5	Subgrade
B-029-0-21	40.110499	-82.973656	929.0	Ramp K	16+67	25' RT.	7.5	Subgrade
B-030-0-21	40.110863	-82.974607	937.1	Ramp K	19+68	27' RT.	40.0	Bridge / Subgrade
B-031-0-21	40.111100	-82.975308	940.6	Ramp K	21+80	11' LT.	40.0	Bridge / Subgrade
B-032-0-21	40.111605	-82.976079	938.9	Ramp K	24+63	1' RT.	7.5	Subgrade
B-033-0-21	40.111894	-82.976618	913.3	Ramp K	26+46	10' RT.	119.7	Bridge / Subgrade
B-034-0-21	40.112095	-82.977907	926.7	Ramp K	30+16	2' LT.	11.5	Subgrade
B-035-0-21	40.111794	-82.979258	915.6	Ramp K	34+09	22' RT.	7.5	Subgrade
B-036-0-21	40.110627	-82.974843	916.2	Ramp O	19+28	29' RT.	7.5	Subgrade
B-037-0-21	40.111592	-82.975360	926.4	Ramp O	23+07	2' RT.	7.5	Subgrade
B-038-0-21	40.112234	-82.976395	912.8	Ramp O	26+82	1' LT.	120.0	Bridge / Subgrade
B-039-0-21	40.112423	-82.978119	929.8	Ramp O	31+80	5' LT.	7.5	Subgrade
B-040-0-21	40.112212	-82.979023	925.8	Ramp O	34+43	20' RT.	7.5	Subgrade
B-041-0-21	40.111728	-82.979914	921.7	Ramp O	37+47	58' LT.	7.5	Subgrade
B-042-0-21	40.112634	-82.975132	910.3	Ramp N	141+47	31' RT.	7.5	Subgrade
B-043-0-21	40.113588	-82.975142	906.0	Ramp N	145+00	11' RT.	7.5	Subgrade

Borings were drilled using a either CME 45B, CME 55T or CME 75T truck-mounted or track-mounted drilling rig utilizing 3.25-inch (inner diameter) hollow stem augers. Soil samples for subgrade borings were typically recovered continuously to a depth of 7.5 ft bgs, each using an 18-inch split spoon sampler

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(AASHTO T-206 “Standard Method for Penetration Test and Split Barrel Sampling of Soils.”). The soil samples obtained from the exploration program were visually observed in the field by the NEAS field representative and preserved for review by a Geologist for possible laboratory testing. Standard penetration tests (SPT) were conducted using CME auto hammer that has been calibrated to be 81.7%, 63.4%, 79% efficient (depending on the specific rig used and the calibration date of the hammer) as indicated on the boring logs (Appendix B).

Field boring logs were prepared by drilling personnel and included pavement description (where present), lithological description, SPT results recorded as blows per 6-inch increment of penetration and estimated unconfined shear strength values on specimens exhibiting cohesion (using a hand penetrometer). Groundwater level observations were recorded both during and after the completion of drilling. These groundwater level observations are included on the individual boring logs (provided in Appendix B). After completing the borings, the boreholes were backfilled with either auger cuttings, bentonite chips, or a combination of these materials and patched accordingly with the cold patch asphalt and/or cement when drilling through the roadway.

3.2. Pavement Coring Exploration Program

The pavement coring investigation program for the project was conducted by NEAS concurrently with the subgrade exploration on April 21, 2021 and included a total of seven (7) pavement cores. As described in Section 3.1. of this report, the indicated target boring locations were located in the field by NEAS prior to drilling utilizing handheld GPS equipment in areas that were not restricted by maintenance of traffic efforts or utilities. Measurements, location information, photographs and other details of each core sample can be found in the Pavement Core Logs included within Appendix B, and are summarized in Table 2 below. The approximate location for each core is depicted on the Boring Location Plan provided in Appendix A.

Cores were drilled using a portable, truck-mounted, electric powered coring drill with a 4-inch (outer diameter) diamond tipped drill bit and utilizing water as the circulating fluid. Asphalt and concrete thicknesses were measured in the field after the cores were extracted and down-hole measurements were made. Each core sample was then photographed, logged, and placed in a core box for transportation to NEAS’s laboratory. Following field documentation, photographs and borehole completion, the core hole was backfilled to existing grade with either asphalt patch or quick-set concrete (where appropriate). Once in the laboratory the cores were: 1) re-measured for thickness verification and photographed; 2) checked for composition; and 3) reviewed for individual layer identification and subsequent measurements.

Table 2: Pavement Core Summary

Pavement Core Number	Latitude	Longitude	Elevation (NAVD 88) (ft)	Alignment	Station	Offset	Length (in)
X-001-0-21	40.107653	-82.979486	912.3	Ramp P	1014+17	46' RT.	18.75
X-002-0-21	40.113628	-82.975595	909.1	I-71	144+42	23' LT.	16.75
X-003-0-21	40.120010	-82.973021	886.2	I-71	168+85	28' LT.	16.50
X-004-0-21	40.125285	-82.972180	895.0	I-71	188+28	33' LT.	17.00
X-005-0-21	40.111052	-82.976900	914.1	I-71	134+39	49' RT.	16.50
X-006-0-21	40.118246	-82.973248	891.0	I-71	162+45	56' RT.	18.00
X-007-0-21	40.125854	-82.971865	896.6	I-71	190+40	44' RT.	14.75

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3.3. Retaining Wall Exploration Program

Between October 23 and October 25, 2023, NEAS also conducted six project borings for two planned retaining walls, which were subsequently eliminated from the design. The boring logs can be found in Appendix B.

3.4. Laboratory Testing Program

The laboratory testing program consisted of classification testing, moisture content determinations and sulfate content testing. Data from the laboratory testing program were incorporated onto the boring logs (Appendix B). Soil samples are retained at the laboratory for 60 days following report submittal, after which time they will be discarded.

3.4.1. Classification Testing

Representative soil samples were selected for index property (Atterberg Limits) and gradation testing for classification purposes on approximately 50% of the samples. At each boring location, the upper two samples obtained below the proposed top of subgrade elevation were generally tested while additional samples were selected for testing with the intent of properly classifying the subsurface soil and groundwater conditions within the planned project limits. Soils not selected for testing were compared to laboratory tested samples/strata and classified visually. Moisture content testing was conducted on all samples. The laboratory testing was performed in general accordance with applicable AASHTO specifications and ODOT Supplements.

Final classification of soil strata in accordance with AASHTO M-145 “Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes,” as modified by ODOT “Classification of Soils” was made once laboratory test results became available. The results of the soil classification are presented on the boring logs in Appendix B.

3.4.2. Standard Penetration Test Results

Standard Penetration Tests (SPT) and split-barrel (commonly known as split-spoon) sampling of soils were performed continuously in the project borings performed. To account for the high efficiency (automatic) hammers used during SPT sampling, field SPT N-values were converted based on the calibrated efficiency (energy ratio) of the specific drill rig's hammer. Field N-values were converted to an equivalent rod energy of 60% (N_{60}) for use in analysis or for correlation purposes. The resulting N_{60} values are shown on the boring logs provided in Appendix B.

3.4.3. Sulfate Testing

Sulfate testing was generally performed on one sample for each subgrade or roadway boring performed for pavement/subgrade design purposes. The selected samples were tested in accordance with ODOT Supplement 1122, “Determining Sulfate Content in Soils” dated July 17, 2015. In general, the upper most sample (within 3 ft of the proposed subgrade elevation) from each boring was tested when feasible. Testing results are summarized in ODOT Sulfate Supplement 1122 Table within Appendix C.

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The subsurface conditions encountered during NEAS's explorations are described in the following subsections and/or on each boring log presented in Appendix B. The boring logs represent NEAS's interpretation of the subsurface conditions encountered at each boring location based on our site observations, field logs, visual review of the soil samples by NEAS's geologist, and laboratory test results. The lines designating the interfaces between various soil strata on the boring logs represent the approximate interface location; the actual transition between strata may be gradual and indistinct. The subsurface soil and groundwater characterizations included herein, including summary test data, are based on the subsurface findings from the geotechnical explorations performed by NEAS as part of the referenced project. At the time of the composition of this report, pavement grade information has been assumed to be consistent with project profile basemap provided by TranSystems dated July 12, 2021. It should be noted that for the purposes of this report and our analysis the term 'subgrade' has been assumed to represent soils and/or soil conditions from 1.5 ft below proposed final pavement grades to a depth of 7.5 ft below the proposed pavement grades.

4.1. Existing Pavement

The pavement section thickness in terms of asphalt, concrete, and granular base was measured at representative subgrade borings and pavement cores. Pavement section thicknesses were measured during the subsurface exploration and are recorded on the test boring log and pavement core provided in Appendix B. A summary of these measurements is provided in Table 3 and 4 below.

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Table 3: Measured Pavement Thicknesses Based on Subgrade Borings

Boring ID	Alignment	Asphalt Thickness (in)	Concrete Thickness (in)	Base thickness (in)	Total thickness (in)
B-001-0-21	I-71	17.0	-	7.0	24.0
B-002-0-21	I-71	16.0	-	8.0	24.0
B-003-0-21	I-71	16.0	-	8.0	24.0
B-004-0-21	I-71	17.0	-	7.0	24.0
B-005-0-21	I-71	17.0	-	7.0	24.0
B-006-0-21	I-71	19.0	-	7.0	26.0
B-007-0-21	I-71	17.0	-	7.0	24.0
B-008-0-21	I-71	16.0	-	7.0	23.0
B-009-0-21	I-71	18.0	-	7.0	25.0
B-010-0-21	I-71	18.0	-	7.0	25.0
B-011-0-21	I-71	18.0	-	18.0	36.0
B-012-0-21	I-71	18.0	-	6.0	24.0
B-013-0-21	I-71	15.0	-	7.0	22.0
B-014-0-21	I-71	14.0	-	6.0	20.0
B-015-0-21	I-71	15.0	-	7.0	22.0
B-016-0-21	I-71	15.0	-	6.0	21.0
B-017-0-21	I-71	14.5	-	6.0	20.5
B-018-0-21	Ramp M	17.0	-	6.0	23.0
B-019-0-21	Ramp M	16.0	-	7.0	23.0
B-020-0-21	Ramp M	16.0	-	6.0	22.0
B-021-0-21	Ramp M	18.0	-	6.0	24.0
B-022-0-21	Ramp M	18.0	-	7.0	25.0
B-023-0-21	Ramp M	18.0	-	5.0	23.0
B-024-0-21	Ramp M	18.0	-	7.0	25.0
B-025-0-21	Ramp P	17.0	-	8.0	25.0
B-026-0-21	Ramp P	16.0	-	9.0	25.0
B-027-0-21	Ramp P	16.0	-	8.0	24.0
B-028-0-21	Ramp K	9.0	9.0	5.0	23.0
B-029-0-21	Ramp K	8.0	9.0	5.0	22.0
B-030-0-21	Ramp K	11.0	-	6.0	17.0
B-031-0-21	Ramp K	12.0	-	5.0	17.0
B-032-0-21	Ramp K	9.0	-	6.0	15.0
B-033-0-21	Ramp K	6.0	7.0	5.0	18.0
B-034-0-21	Ramp K	8.0	-	6.0	14.0
B-035-0-21	Ramp K	9.0	-	7.0	16.0
B-036-0-21	Ramp O	8.0	-	8.0	16.0
B-037-0-21	Ramp O	10.0	-	7.0	17.0
B-038-0-21	Ramp O	18.0	-	6.0	24.0
B-039-0-21	Ramp O	8.0	-	7.0	15.0
B-040-0-21	Ramp O	4.0	8.0	6.0	18.0
B-041-0-21	Ramp O	4.0	8.0	6.0	18.0
B-042-0-21	Ramp N	12.0	-	6.0	18.0
B-043-0-21	Ramp N	12.0	-	8.0	20.0

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Table 4: Measured Pavement Thicknesses Based on Pavement Cores

Core ID	Proposed Alignment	Top Layer Asphalt Thickness (in)	Second Layer Asphalt Thickness (in)	Third Layer Asphalt Thickness (in)	Fourth Layer Asphalt Thickness (in)	Total Asphalt Thickness (in)
X-001-0-21	Ramp P	10.00	8.75	-	-	18.75
X-002-0-21	I-71	3.50	3.00	10.25	-	16.75
X-003-0-21	I-71	1.50	15.00	-	-	16.50
X-004-0-21	I-71	3.50	5.00	8.50	-	17.00
X-005-0-21	I-71	1.50	15.00	-	-	16.50
X-006-0-21	I-71	2.00	3.25	4.25	8.50	18.00
X-007-0-21	I-71	3.50	4.50	6.75	-	14.75

4.2. Subgrade Conditions

The subgrade conditions in the project area are relatively consistent and are generally comprised of primarily cohesive natural overburden soils (A-4a, A-6a, A-6b, and A-7-6) and minor non-cohesive overburden soils (A-1-a, A-1-b, A-2-4). With respect to sulfate within the subgrade soil, based on the project laboratory testing program, three soil samples present a sulfate content value larger than 5,000 ppm within all the project borings performed.

The following subsections present a brief summary of the subsurface conditions by ramp/roadway segment with problem areas highlighted where present.

4.2.1. I-71

The subgrade soils encountered along I-71 consisted of 84% cohesive materials and 15% granular materials. Those cohesive materials are: 1) Sandy Silt (A-4a, 49%); 2) Silt and Clay (A-6a, 22% of samples); 3) Silty Clay (A-6b, 8% of samples); and 4) Clay (A-7-6, 5% of samples). With respect to the consistency of the fine-grained soils, the descriptions varied from stiff to hard correlating to N_{60} values between 10 and 41 bpf and hand penetrometer readings between 1.50 tsf and over 4.5 tsf. Natural moisture contents ranged from 5 to 28 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 22 to 39 percent and from 14 to 25 percent, respectively.

Sixteen percent (16%) of the samples taken along the I-71 were classified as non-cohesive soils and were comprised of: 1) Gravel and Stone Fragments with Sand (A-1-b, 10% of samples); 2) Stone Fragments (A-1-a, 3% of samples); and 3) Stone Fragments with Sand and Silt (A-2-4, one sample). With respect to the relative compactness of the coarse-grained soils, the descriptions varied from loose to very dense correlating to converted SPT-N values (N_{60}) values between 7 and 109 blows per foot (bpf). Natural moisture content ranged from 4 to 24 percent.

4.2.1. Ramp K

The subgrade soils encountered along Ramp K consisted of 93% cohesive materials and 7% granular materials. Those cohesive materials are: 1) Silty Clay (A-6b, 46% of samples); 2) Silt and Clay (A-6a, 32% of samples); and 3) Sandy Silt (A-4a, 14%). With respect to the consistency of the fine-grained soils, the descriptions varied from stiff to hard correlating to N_{60} values between 3 and 28 bpf. Natural moisture contents ranged from 10 to 26 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 24 to 37 percent and from 14 to 22 percent, respectively.

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Seven percent (7%) of the samples taken along Ramp K were classified as non-cohesive soils and were comprised of Gravel and Stone Fragments with Sand (A-1-b, 7% of samples). With respect to the relative compactness of the coarse-grained soils, the descriptions varied from loose to medium dense correlating to converted SPT-N values (N_{60}) values between 5 and 12 blows per foot (bpf). Natural moisture content was 7 percent.

4.2.2. *Ramp M*

The subgrade soils encountered along Ramp M consisted of 89% cohesive materials and 11% granular materials. Those cohesive materials are: 1) Silt and Clay (A-6a, 36% of samples); 2) Silty Clay (A-6b, 29% of samples); 3) Clay (A-7-6, 11% of samples) and 4) Sandy Silt (A-4a, 14%). With respect to the consistency of the fine-grained soils, the descriptions varied from stiff to hard correlating to N_{60} values between 10 and 34 bpf and hand penetrometer reading ranging from 1.50 tsf to 4.50 tsf. Natural moisture contents ranged from 9 to 27 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 25 to 50 percent and from 16 to 22 percent, respectively.

Eleven percent (11%) of the samples taken along Ramp M were classified as non-cohesive soils and were comprised of: 1) Gravel and Stone Fragments with Sand (A-1-b, 7% of samples); and 2) Stone Fragments (A-1-a, 4% of samples). With respect to the relative compactness of the coarse-grained soils, the descriptions varied from loose to medium dense correlating to converted SPT-N values (N_{60}) values between 10 and 16 blows per foot (bpf). Natural moisture content ranged from 6 to 11 percent.

4.2.3. *Ramp N*

The subgrade soils encountered along Ramp N consisted of 88% cohesive materials and 12% granular materials. Those cohesive materials are: 1) Silt and Clay (A-6a, 50% of samples); 2) Sandy Silt (A-4a, 25%); and 3) Clay (A-7-6, 13% of samples). With respect to the consistency of the fine-grained soils, the descriptions varied from medium stiff to very stiff correlating to N_{60} values between 8 and 27 bpf and hand penetrometer readings ranging from 2.75 tsf to over 4.50 tsf. Natural moisture contents ranged from 10 to 28 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 25 to 53 percent and from 17 to 25 percent, respectively.

Twelve percent (12%) of the samples taken along Ramp N were classified as non-cohesive soils and were comprised of: 1) Stone Fragments with Sand and Silt (A-2-4, one sample). With respect to the relative compactness of the coarse-grained soils, the description was medium dense correlating to a converted SPT-N value (N_{60}) of 20 blows per foot (bpf). Natural moisture content was 7 percent.

4.2.4. *Ramp O*

The subgrade soils encountered along Ramp O consisted of 100% cohesive materials. Those cohesive materials are: 1) Silt and Clay (A-6a, 50% of samples); and 2) Sandy Silt (A-4a, 50%). With respect to the consistency of the fine-grained soils, the descriptions varied from stiff to hard correlating to N_{60} values between 8 and 26 bpf and hand penetrometer readings ranging from 4.00 tsf to over 4.50 tsf. Natural moisture contents ranged from 8 to 14 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 23 to 32 percent and from 13 to 17 percent, respectively.

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4.2.1. Ramp P

The subgrade soils encountered along Ramp P consisted of 87% cohesive materials and 15% granular materials. Those cohesive materials are: 1) Sandy Silt (A-4a, 29%); 2) Silt and Clay (A-6a, 29% of samples); and 3) Silty Clay (A-6b, 29% of samples). With respect to the consistency of the fine-grained soils, the descriptions varied from stiff to hard correlating to N_{60} values between 5 and 35 bpf and hand penetrometer readings ranging from 2.50 tsf to 4.50 tsf. Natural moisture contents ranged from 10 to 19 percent. Based on Atterberg Limit tests performed on representative samples of the fine-grained subgrade soils obtained within these pavement widening limits, the liquid and plastic limits ranged from 24 to 39 percent and from 15 to 20 percent, respectively.

Thirteen percent (13%) of the samples taken along Ramp P were classified as non-cohesive soils and were comprised of: 1) Stone Fragments with Sand and Silt (A-2-4, one sample); 2) Gravel and Stone Fragments with Sand (A-1-b, 7% of samples); and 3) Stone Fragments (A-1-a, one sample). With respect to the relative compactness of the coarse-grained soils, the description ranged from loose to medium dense correlating to converted SPT-N values (N_{60}) value between 7 and 16 blows per foot (bpf). Natural moisture content ranged from 6 to 11 percent.

4.2.2. Groundwater

Groundwater was encountered only in boring B-033-0-21 during drilling in all the project borings performed as part of the referenced project. Based on measurements at this boring location groundwater was encountered at a depth of 18.0 ft bgs (elevation 895.3 ft amsl). It should be noted that groundwater is affected by many hydrologic characteristics in the area and may vary from those measured at the time of the exploration.

4.2.3. Bedrock

Bedrock was not encountered in all the project borings within the subgrade depth.

5. ANALYSES AND RECOMMENDATIONS

We understand that the project FRA-71/270-28.27/25.99A consists of widening IR-71 NB at the IR-270 and IR-71 interchange, as well as pavement replacement of five associated ramps. For this purpose, a roadway exploration and subsequent analysis was completed for the referenced project. The analysis completed for the proposed project included a subgrade (GB1) analysis. The subgrade analysis was performed in accordance with ODOT's GB1 criteria utilizing the ODOT provided *GB1: Subgrade Analysis Spreadsheet* (GB1_SubgradeAnalysis.xls, Version 14.6 dated February 11, 2022). Input information for the spreadsheet was based on the soil characteristics gathered during NEAS's subgrade exploration (i.e., SPT results, laboratory test results, etc.). A GB1 analysis was performed for each of the referenced pavement widening areas individually.

Based on our evaluation of the subsurface conditions and our geotechnical engineering analyses of the proposed project, it is our opinion that the subgrade conditions encountered are generally satisfactory and pavement can be designed without the need for extreme levels of remediation, especially with the use of global stabilization per the GB1. In general, the subgrade soils throughout the project will be globally stabilized by either Excavate and Replace (Item 204 with Geotextile) or chemical stabilization. The following sections provide further detail about the analysis performed and the recommended remediation.

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5.1. Subgrade Analysis

A GB1 analysis was performed to identify the method, location, and dimensions (including depth) of required subgrade stabilization for the project. In addition to identifying stabilization recommendations, pavement design parameters are also determined to aid in pavement section design. The subsections below present the results of our GB1 analysis including pavement design parameters and unsuitable and/or unstable subgrade conditions if any are identified within the project limits. A GB1 analysis spreadsheet is provided in Appendix C.

5.1.1. Pavement Design Recommendations

A GB1 analysis was performed using the subgrade soil data obtained during our field exploration program to evaluate the soil characteristics and develop pavement parameters for use in pavement design. The subgrade analysis parameters recommended for use in pavement design are presented in Table 5 below. Provided in the table are ranges of maximum, minimum and average N_{60L} values for the indicated segments as well as the design CBR value recommended for use in pavement design.

Table 5: Pavement Design Values

Section	Maximum N_{60L}	Minimum N_{60L}	Average N_{60L}	Average PI Values	Design CBR
Entire Project	26	3	14	12	7
IR-71 NB	25	10	16	10	7
IR-71 NB Sta. 156+67 to End	20	11	16	9	8
Ramp K	16	3	11	15	6
Ramp M	22	10	14	18	6
Ramp N	25	8	18	14	7
Ramp O	26	8	15	11	7
Ramp P	22	5	12	14	6

5.1.2. Unstable Subgrade

Per ODOT's GB1, the presence of select subgrade conditions (i.e., unsuitable) are prohibited within the subgrade zone for new pavement construction. These unsuitable subgrade conditions generally include the presence of rock and specific soil types. With respect to the planned roadways, these subgrade conditions are further discussed in the following subsections.

5.1.2.1. Rock

Rock was not encountered in any of the borings performed within the project roadway limits.

5.1.2.2. Prohibited Soils

Unsuitable soil types per the GB1, which include A-4b, A-2-5, A-5, A-7-5, A-8a, A-8b, and soils with liquid limits greater than 65, were not encountered within the subgrade of the referenced project roadway segments.

5.1.3. Unstable Subgrade

The unstable subgrade conditions generally include the presence of weak soil conditions and overly moist soil conditions. With respect to the planned roadway sections, these subgrade conditions are further discussed in the following subsections.

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5.1.3.1. Weak Soils

Soils for which the lowest N_{60} (N_{60L}) at the referenced boring location is less than 12 bpf and in some cases less than 15 bpf (i.e., where moisture content is greater than optimum plus 3 percent), or in which the lowest HP reading at the referenced boring location is less than 1.5 and, in some cases, less than 1.875 (i.e., where moisture content is greater than optimum plus 3 percent), subgrade stabilization depths are recommended per *Figure B - Subgrade Stabilization* within the GB1.

It should be noted that for the purposes of this report the term "weak soils" has been assumed to represent subgrade soils of these conditions. A summary of the boring locations where unstable soils were encountered and determined to have a potential impact on subgrade performance are shown in Table 6 below, per the roadway segment for which they were encountered. Also included is the associated GB1 recommended remediation depth with the method of either excavation and replacement or chemical treatment within the project limits.

Table 6: Unstable Soil Locations Summary

Boring ID	N_{60L}	Subgrade Depth (ft)	Remediation Depth (inches)		
			Excavate and Replace (Item 204 w/ Geotextile)	Excavate and Replace (Item 204 w/ Geogrid - SS 861)	Chemical Stabilization (Item 206)
Ramp K					
B-029-0-21	9	1.4 - 2.9	12	N/A	14
Ramp O					
B-041-0-21	11	1.1 - 2.6	12	N/A	12
Ramp P					
B-026-0-21	5	0.1 - 1.6	21	15	14
B-027-0-21	7	(-)1.1 - 0.4	15	N/A	14

Note: N/A, Not Applicable based on GB1- *Figure B - Subgrade Stabilization*

It should be noted that *Figure B - Subgrade Stabilization* does not apply to soil types A-1-a, A-1-b, A-3, or A-3a, nor to soils with N_{60L} values of 15 or more. Per GB1 guidance, *these soils should be reworked to stabilize the subgrade*.

5.1.3.2. High Moisture Content Soils

High moisture content soils are defined by the GB1 as soils that exceed the estimated optimum moisture content (per *Figure A - Optimum Moisture Content* within the GB1) for a given classification by 3 percent or more. Per the GB1, soils determined to be above the identified moisture content levels are a likely indication of the presence of an unstable subgrade and may require some form of subgrade stabilization. High moisture content soils were encountered along proposed alignments within 3 ft of proposed finished grade of roadway segments. Therefore, remediation is needed for the high moisture content soils encountered at these roadway alignments. Summaries of the boring locations where high moisture content conditions were encountered within the limits of each proposed alignment are shown in Table 7 below.

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Table 7: High Moisture Content Soils Location Summary

Boring ID	High MC Soil Type	Moisture Content (%)	Optimum Moisture Content (%)	Subgrade Depth (ft)
I-71				
B-001-0-21	A-6b	21	16	0.8 - 2.3
B-004-0-21	A-6a	17	14	1.6 - 3.1
B-014-0-21	A-4a	13	10	1.1 - 2.6
Ramp K				
B-029-0-21	A-6a	17	14	1.4 - 2.9
Ramp M				
B-018-0-21	A-7-6	25	19	1.9 - 3.4
Ramp N				
B-004-0-21	A-6a	17	14	0.3 - 1.8
B-005-0-21	A-4a	15	10	1.9 - 3.4
Ramp P				
B-026-0-21	A-6a	17	14	0.1 - 1.6
		19	14	1.6 - 3.1
B-001-0-21	A-6b	21	16	0.7 - 2.2

5.1.4. High Sulfate Content Soils

High sulfate content soils are defined as soils that exceed 3,000 ppm. Where high sulfate content soils are encountered, the GB1 prohibits the use of chemical stabilization without prior consultation with the District Geotechnical Engineer. Three soil samples in borings B-008-0-21, B-014-0-21 and B-016-0-21 present a sulfate content greater than 3,000 ppm, however, less than 5,000 ppm. Three soil samples in borings B-009-0-21, B-010-0-21 and B-040-0-21 present a sulfate content greater than 5,000 ppm.

5.2. Stabilization Recommendations

5.2.1. Subgrade Stabilization

Unstable subgrade conditions, including areas of weak soils and high moisture content soils, were encountered throughout 8 percent of the proposed pavement widening area along IR-71 NB and the IR-71 and IR-270 interchange associated ramps. Three soil samples in borings B-008-0-21, B-014-0-21 and B-016-0-21 present a sulfate content greater than 3,000 ppm, however, less than 5,000 ppm. Three soil samples in borings B-009-0-21, B-010-0-21 and B-040-0-21 present a sulfate content greater than 5,000 ppm. Although these materials were encountered throughout the site, guidance from ODOT's GB1 states that *"For all interstates and other divided highways with four or more lanes more than 1-mile length, the subgrade of the entire project shall be chemically stabilized (global stabilization)"* and therefore global chemical stabilization be performed for the proposed interstate interchange improvement project except where otherwise indicated in this report.

The global chemical stabilization of the referenced IR-71 NB and ramps subgrade soils included within this project, should be performed to a minimum depth of 12 inches utilizing cement as the stabilizing chemical. The stabilization efforts should extend a minimum of 18-inches beyond the edge of the paved roadway, shoulder or median. The mix design should be conducted in accordance with ODOT's CMS Supplement 1120 (Mixture Design for Chemically Stabilized Soils). For design purposes it may be assumed that the cement addition will be 5% using the following formula.

$$\text{Cement: } C = 0.75 \times T \times 115 \times 0.05$$

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Where:

C = amount of chemical in pounds / square yard and

T = thickness of the treatment zone in inches

A dry density of 115-pounds per cubic foot (pcf) is assumed.

For areas where high sulfate content soils were encountered, NEAS discussed our recommendations with the District Geotechnical Engineer on August 17, 2022. It is recommended that Excavate and Replace with geotextile (Item 204) be performed to a depth of 12 inches below the proposed top of subgrade. Our recommended limits for the indicated project subgrade stabilization are provided in Table 8 below.

Table 8: Stabilization Recommendations

Segment	Average N60L	Remediation Depth (inches)		
		Excavate and Replace (Item 204 w/ Geotextile)	Excavate and Replace (Item 204 w/ Geogrid - SS 861)	Chemical Stabilization (Item 206)
Ramps K, M, N, O, P & IR-71 NB from Begin to Sta. 156+67	16	12	N/A	12
IR-71 NB from Sta. 156+67 to End	11	12	N/A	N/A

Note: N/A, Not Applicable based on GB1- Figure B - Subgrade Stabilization

5.3. Embankment Construction Recommendations

In areas where additional embankment material is proposed along existing slopes (i.e., side-hill sliver fills) that are steeper than 8H:1V but flatter than 4H:1V, it is recommended that the proposed embankment be benched into the existing slopes in accordance with Item 203.05 “Embankment Construction Methods” of the ODOT CMS. For areas where additional embankment material is proposed along existing slopes that are steeper than 4H:1V, it is recommended that the proposed embankment be designed and constructed in accordance with ODOT’s GB2. For sidehill fills planned on existing slopes steeper than 4H:1V, ODOT’s GB2 recommends that *the embankment slopes be constructed utilizing special benching in order to blend the new embankment with the existing slope to prevent the development of a weak shear plane at the interface between the proposed fill and existing slope material* (ODOT [2], 2021). As proposed cross-sections are not available at this time, at this stage of the project a special benching scheme similar to that shown in Figures 1, 2 or 3, as appropriate, of the ODOT GB2 should be used in areas where special benching is recommended. The height and width dimensions of the special benching scheme shown in these figures should be arranged to minimize the required cut and fill quantities, though the height of a single bench shall not exceed 20 ft without a stability analysis and design per OSHA requirements. Additionally, it may be appropriate to adjust the bench slope shown from a 1H:1V to a 1.75H:1V slope if the existing slope is made up of primarily granular materials. The benched material should be replaced with compacted engineered fill per Item 203 of the ODOT CMS, while proper lift thicknesses and material density should be maintained in the proposed fill per Item 203.06 of the ODOT CMS. In situations where it is not practical to extend the final bench through the existing roadway due to maintenance of traffic concerns, a benching scheme similar to that shown in Figure 1a of the ODOT GB2 can be used in order to avoid impacting the existing roadway, guardrail or shoulder. This scheme results in the placement of a temporary over-steepened fill that can later be “shaved-off” to bring the slope to the final proposed grade.

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6. STRUCTURE BORING PROFILES

NEAS performed 6 project borings between October 23, 2023, and October 25, 2023 for the two planned retaining walls which later were eliminated. Each individual project boring log (included within Appendix B) includes the recorded boring latitude and longitude location (based on the surveyed Ohio State Plane South, NAD83, location) and the corresponding ground surface elevation.

7. QUALIFICATIONS

This investigation was performed in accordance with accepted geotechnical engineering practice for the purpose of characterizing the subsurface conditions along the referenced portions of roadways. This report has been prepared for TranSystems and ODOT to be used solely in evaluating the subgrade soils within the project limits and presenting geotechnical engineering recommendations specific to this project. The assessment of general site environmental conditions or the presence of pollutants in the soil, rock and groundwater of the site was beyond the scope of this geotechnical exploration. Our recommendations are based on the results of our field explorations, laboratory test results from representative soil samples, and geotechnical engineering analyses. The results of the field explorations and laboratory tests, which form the basis of our recommendations, are presented in the appendices as noted. This report does not reflect any variations that may occur between the borings or elsewhere on the site, or variations whose nature and extent may not become evident until a later stage of construction. In the event that any changes occur in the nature, design or location of the proposed improvement work, the conclusions and recommendations contained in this report should not be considered valid until they are reviewed and have been modified or verified in writing by a geotechnical engineer.

It has been a pleasure to be of service to TranSystems in performing this geotechnical exploration for the FRA-71/270-28.27/25.99A Safety and System Preservation project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

Zhao Mankoci, Ph.D., P.E.
Geotechnical Engineer



Chunmei He
Chunmei (Melinda) He, Ph.D., P.E.
Project Manager

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APPENDIX A

SOIL BORING LOCATION PLAN

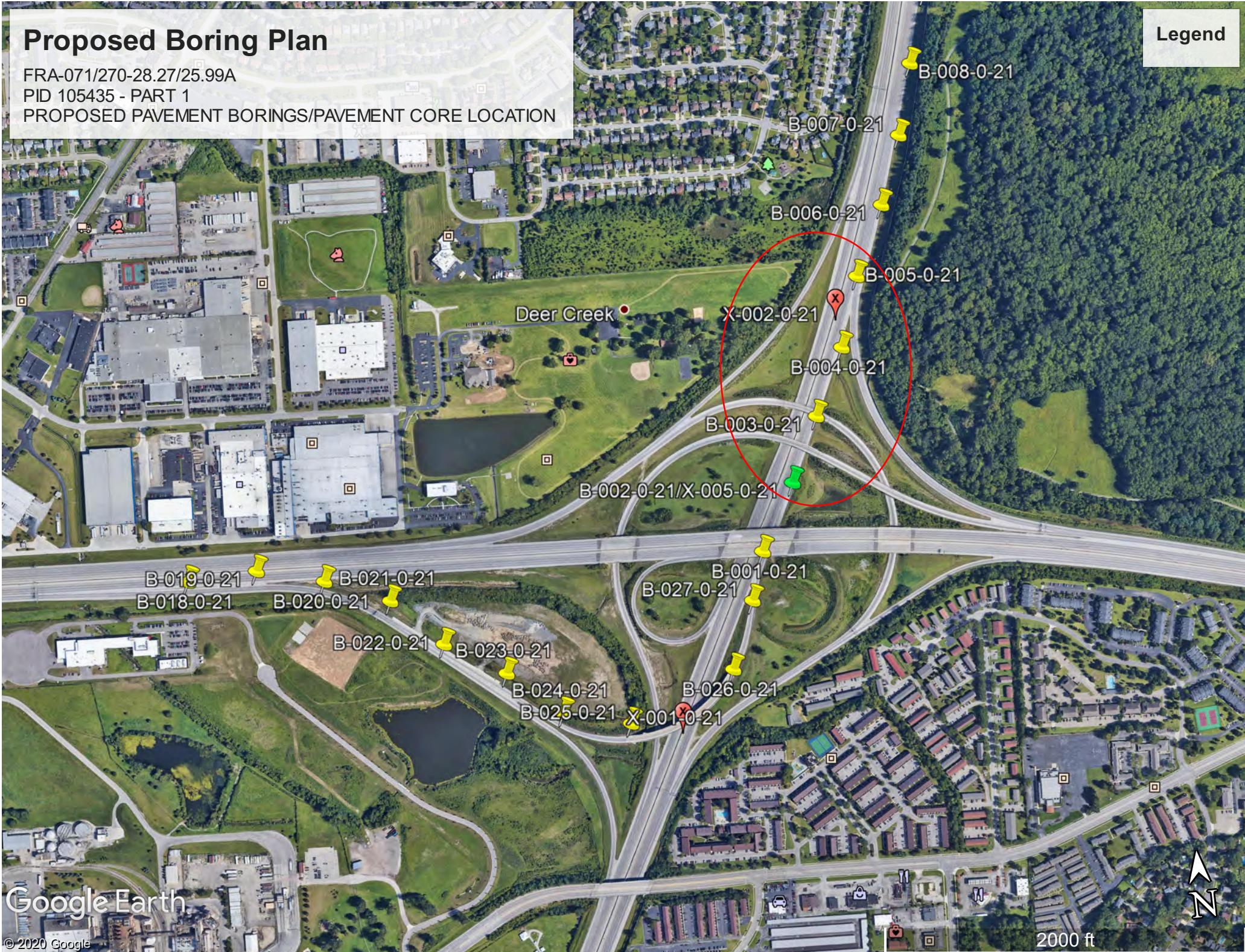
Proposed Boring Plan

FRA-071/270-28.27/25.99A

PID 105435 - PART 1

PROPOSED PAVEMENT BORINGS/PAVEMENT CORE LOCATION

Legend



Google Earth

N

2000 ft

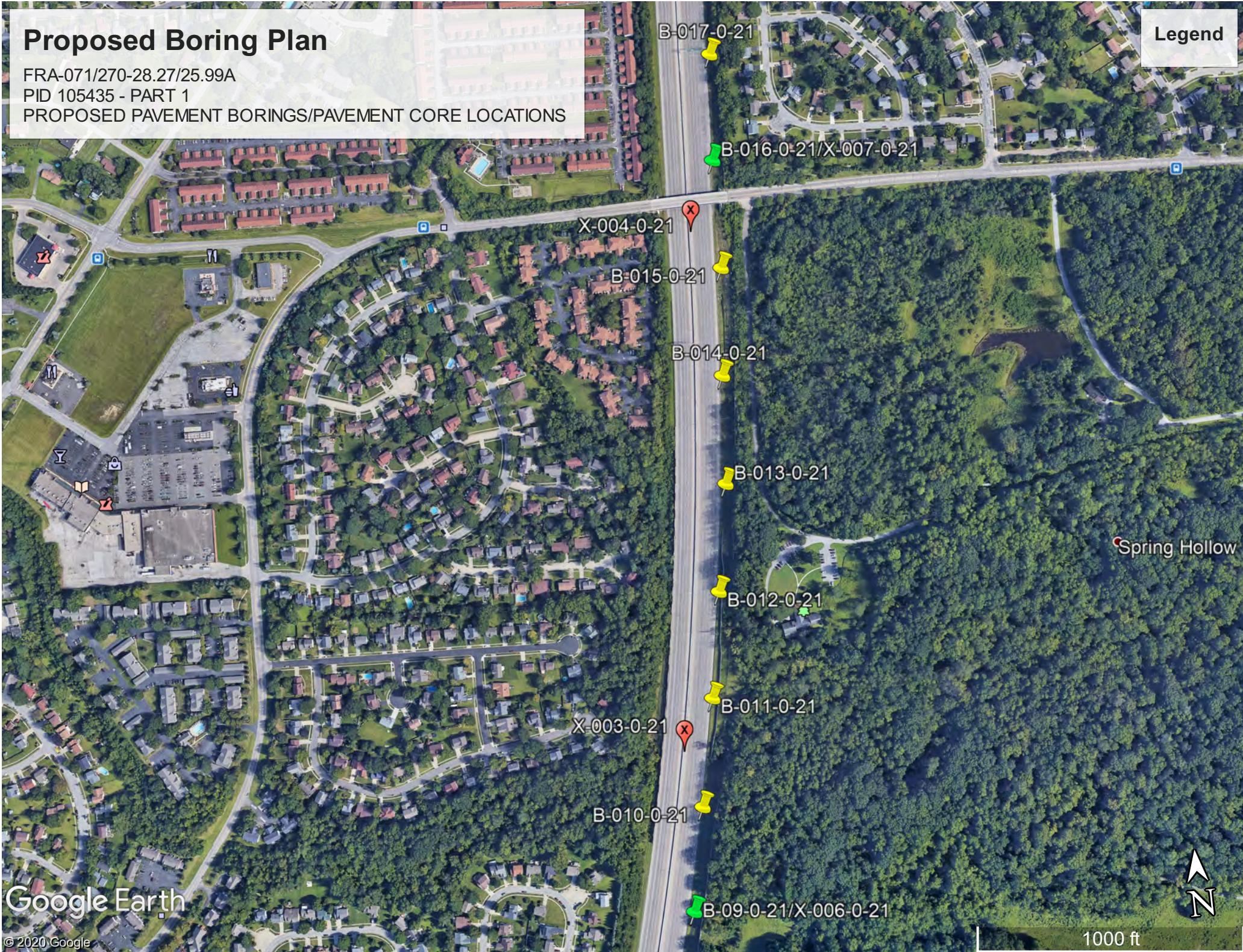
Proposed Boring Plan

FRA-071/270-28.27/25.99A

PID 105435 - PART 1

PROPOSED PAVEMENT BORINGS/PAVEMENT CORE LOCATIONS

Legend



Part 2 Boring Plan

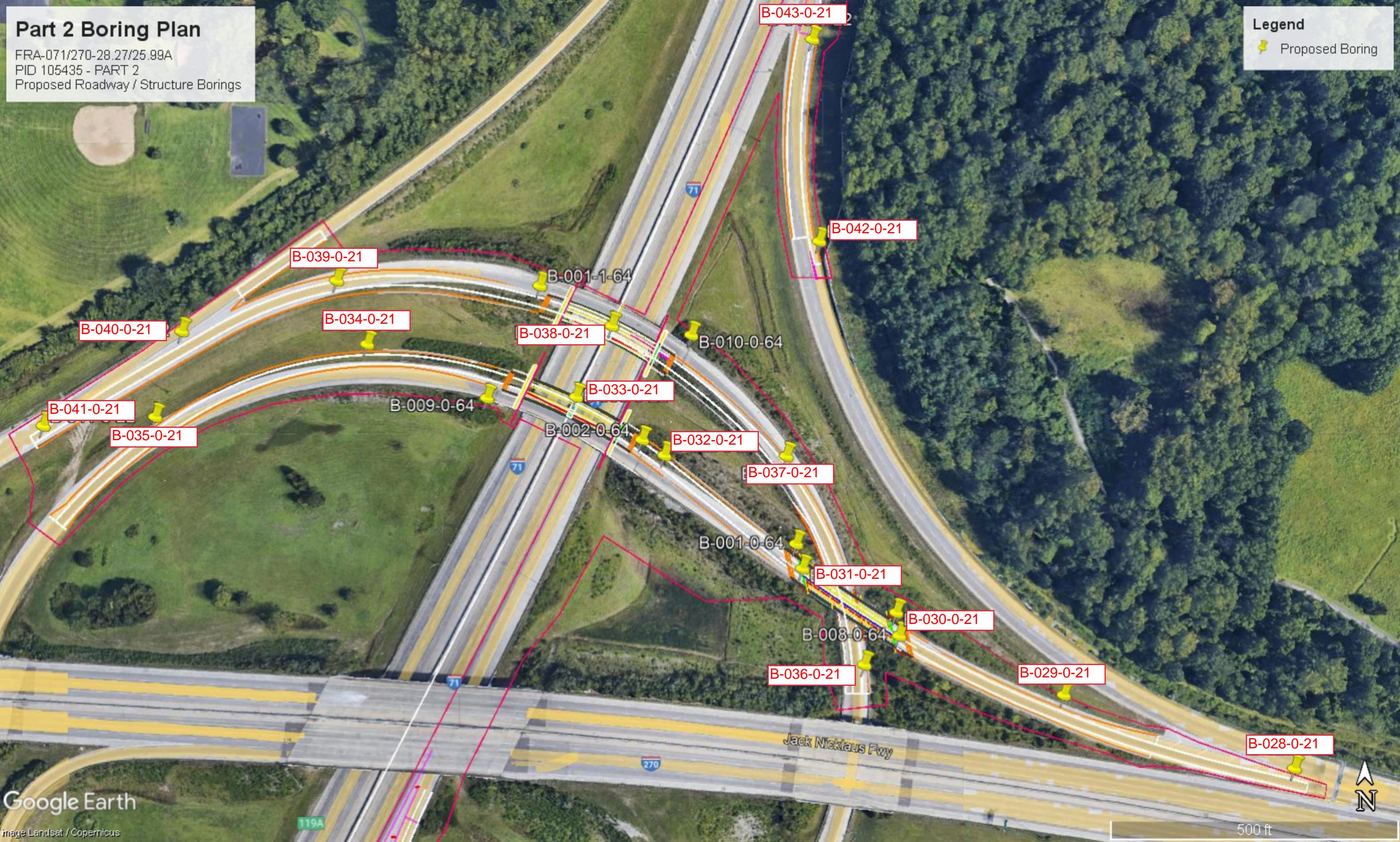
FRA-071/270-28.27/25.99A

PID 105435 - PART 2

Proposed Roadway / Structure Borings

Legend

Proposed Boring



Google Earth

Image Landsat / Copernicus

500 ft

APPENDIX B

BORING LOGS AND TEST RESULTS

LEGEND

SYMBOL	DESCRIPTION	ODOT CLASSIFICATION	SYMBOL	DESCRIPTION	ODOT CLASSIFICATION
	Gravel and/or Stone Fragments	A-1-a		Shale	Visual
	Gravel and/or Stone Fragments with Sand	A-1-b		Weathered Shale	Visual
	Fine Sand	A-3		Sandstone	Visual
	Coarse and Fine Sand	A-3a			
	Gravel and/or Stone Fragments with Sand and Silt	A-2-4 A-2-5		GRADATION (%)	
	Gravel and/or Stone Fragments with Sand, Silt and Clay	A-2-6 A-2-7		GR Gravel CS Coarse Sand MS Medium Sand FS Fine Sand SI Silt CL Clay (<5 micron)	
	Sandy Silt	A-4a			
	Silt	A-4b		SAMPLER SYMBOLS	
	Elastic Silt and Clay	A-5		Shelby Tube	
	Silt and Clay	A-6a		Rock Core	
	Silty Clay	A-6b		Split Spoon Sample (SS)	
	Elastic Clay	A-7-5		*	Indicates a Sample Taken Within 3 ft of Proposed Grade
	Clay	A-7-6			
	Organic Silt	A-8a			
	Organic Clay	A-8b			

ABBREVIATIONS

LL	LIQUID LIMIT (%)	HP	HAND PENETROMETER
PI	PLASTIC INDEX (%0)	PID	PHOTOIONIZATION DETECTOR
WC	MOISTURE CONTENT (%)	UC	UNCONFINED COMPRESSION
SPT	STANDARD PENETRATION TEST	ppm	PARTS PER MILLION
NP	NON PLASTIC	W	WATER FIRST ENCOUNTERED
-200	PERCENT PASSING NO. 200 SIEVE	▼	WATER LEVEL UPON COMPLETION
N ₆₀	ADJUSTED SPT RESULT		
EOB	END OF BORING		

MATERIAL CLASSIFIED BY VISUAL INSPECTION

Sod and Topsoil
 Pavement or Base
 Concrete

Uncontrolled Fill (Describe)

Bouldery Zone

Peat, S-Sedimentary
W-Woody F-Fibrous
L-Loamy & etc

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 134+73, 77' RT.			EXPLORATION ID B-002-0-21								
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: I-71											
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 914.1 (MSL) EOB: 7.5 ft.			PAGE								
START: 4/20/21	END: 4/20/21	SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.111052, -82.976900			1 OF 1								
MATERIAL DESCRIPTION AND NOTES			ELEV. 914.1	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL		
16.0" ASPHALT AND 8.0" BASE (DRILLERS DESCRIPTION)					1														
HARD, BROWNISH GRAY, SANDY SILT , SOME CLAY, TRACE GRAVEL, DAMP (FILL)			912.1		2	13	12	8	SS-1	4.50	6	11	17	39	27	27	17	10	400
HARD, BROWNISH GRAY WITH TRACE BLACK MOTTLES, SILT AND CLAY , SOME SAND, LITTLE GRAVEL, DAMP TO MOIST (FILL)			911.1		3														
VERY STIFF, BROWN WITH TRACE GRAY MOTTLES, CLAY , SOME SILT, LITTLE SAND, LITTLE GRAVEL, CONTAINS IRON STAINING AND BRICK FRAGMENTS, DAMP (FILL)			908.1		4	7	7	19	SS-2	4.50	16	14	13	35	22	28	17	11	-
			906.6		5	3	6	15	SS-3	4.50	-	-	-	-	-	-	-	19	A-6a (V)
					6	3	5	11	SS-4	2.75	-	-	-	-	-	-	-	21	A-7-6 (V)
					7	3	5	56											-
					EOB														
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																			
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																			

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 138+60, 75' RT.			EXPLORATION ID B-003-0-21									
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: I-71												
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 913.7 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1									
START: 4/20/21	END: 4/20/21	SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.112010, -82.976279												
MATERIAL DESCRIPTION AND NOTES			ELEV. 913.7	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL			
16.0" ASPHALT AND 8.0" BASE (DRILLERS DESCRIPTION)				911.7	1															
MEDIUM DENSE TO DENSE, GRAY, STONE FRAGMENTS, LITTLE SAND, TRACE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, DAMP (FILL)					2	9	14 12	35	56	SS-1	-	-	-	-	-	-	9	A-1-a (V)	-	
VERY STIFF, BROWN AND GRAY, SILT AND CLAY, SOME SAND, LITTLE GRAVEL, DAMP TO MOIST					3	9	10 12	30	44	SS-2	-	77	10	5	6	2	NP	NP	NP	
					4	9	6 12	18	67	SS-3	2.75	14	8	15	34	29	32	18	14	
					5	6	6 7	18	67	SS-4	2.25	-	-	-	-	-	-	21	A-6a (V)	100
					6	6	7	22	33											
					7	9														
					EOB															
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																				
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																				

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVELED SOIL CUTTINGS

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. LONG	DRILL RIG: CME 45B	STATION / OFFSET: 146+32, 58' RT.	EXPLORATION ID B-005-0-21															
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. LONG	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 908.2 (MSL) EOB: 7.5 ft.	PAGE															
START: 4/20/21 END: 4/20/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	LAT / LONG: 40.113975, -82.975211	1 OF 1															
MATERIAL DESCRIPTION AND NOTES	ELEV. 908.2	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				
17.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)																			
DENSE, GRAY, STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, DAMP (FILL)		906.2		1															
VERY STIFF TO HARD, BROWN AND GRAY, SANDY SILT, SOME CLAY, LITTLE GRAVEL, SS-3 CONTAINS 1.5" STONE FRAGMENTS, DAMP		905.2		2	45	35	100	SS-1	-	-	-	-	-	-	-	8	A-1-b (V)	-	
				3	15	11													
				4	9	8	23	SS-2	4.50	14	13	17	34	22	26	17	9	A-4a (4)	260
				5	8	7	20	SS-3	4.00	-	-	-	-	-	-	-	15	A-4a (V)	-
				6	10	8	23	SS-4	4.25	-	-	-	-	-	-	-	12	A-4a (V)	-
				7	8	9													
			EOB																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																			
ABANDONMENT METHODS. MATERIALS. QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVELLED SOIL CUTTINGS																			

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 45B			STATION / OFFSET: 150+94, 91' RT.			EXPLORATION ID B-006-0-21										
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: I-71													
PID: 105435 SFN:		DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 903.0 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1										
START: 4/14/21 END: 4/14/21		SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.115133, -82.974503													
MATERIAL DESCRIPTION AND NOTES			ELEV. 903.0	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL				
19.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)					1																
HARD, GRAY, SANDY SILT, LITTLE TO SOME CLAY, TRACE TO SOME GRAVEL, DAMP			900.8		2	8	7	18	100	SS-1	4.50	10	13	18	35	24	24	16	8		
DENSE, GRAY, STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, DAMP			898.5		3	10	6	30	67	SS-2	4.50	27	13	16	27	17	25	16	9		
HARD, GRAY, SANDY SILT, SOME CLAY, SOME GRAVEL, DAMP			897.0		4	4	18			SS-3	-	-	-	-	-	-	-	-	9		
			895.5		5	12	12	37	44	SS-4	4.50	-	-	-	-	-	-	-	10		
					6	10	15														
					7	14	16	41	39												
					EOB																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																					
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																					

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 45B	STATION / OFFSET: 154+66, 89' RT.	EXPLORATION ID B-007-0-21															
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 899.4 (MSL)	PAGE															
START: 4/14/21 END: 4/14/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	EOB: 7.5 ft.	1 OF 1															
MATERIAL DESCRIPTION AND NOTES	ELEV. 899.4	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				

17.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)

MEDIUM DENSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, DAMP (FILL)

VERY STIFF TO HARD, BROWN AND GRAY, SANDY SILT, SOME GRAVEL, LITTLE CLAY, DAMP

Layer	Description	Top Depth	Bottom Depth
1	17.0" Asphalt and 7.0" Base (Drillers Description)	899.4	897.4
2	MEDIUM DENSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, DAMP (FILL)	897.4	896.4
3	VERY STIFF TO HARD, BROWN AND GRAY, SANDY SILT, SOME GRAVEL, LITTLE CLAY, DAMP	896.4	891.9
8	EOB	891.9	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 45B	STATION / OFFSET: 166+56, 86' RT.	EXPLORATION ID: B-010-0-21																
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																	
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 887.8 (MSL)	EOB: 7.5 ft.																
START: 4/14/21 END: 4/14/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	LAT / LONG: 40.119281, -82.972942	PAGE 1 OF 1																
MATERIAL DESCRIPTION AND NOTES	ELEV. 887.8	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL	
								GR	CS	FS	SI	CL	LL	PL	PI					
18.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)				1																
VERY DENSE, BLACK AND BROWN, STONE FRAGMENTS, SOME SAND, TRACE SILT, TRACE CLAY, CONTAINS ASPHALT FRAGMENTS, DAMP (FILL)		885.7		2	13	52	33	SS-1	-	-	-	-	-	-	-	5	A-1-a (V)	-		
		884.8		3	10	28														
HARD, BROWN, SANDY SILT, LITTLE GRAVEL, LITTLE CLAY, CONTAINS TRACE IRON STAINING, DAMP		881.8		4	6	20	56	SS-2	4.50	16	16	19	35	14	33	25	8	A-4a (3)	5767	
		880.3		5	9	6														
VERY STIFF, BROWN AND BROWNISH GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, MOIST		EOB		6	8	27	44	SS-3	4.50	-	-	-	-	-	-	-	12	A-4a (V)	-	
				7	12	5	6	SS-4	3.50	4	7	15	43	31	33	19	14	20	A-6a (9)	-
				13																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																				
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVELLED SOIL CUTTINGS																				

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 45B	STATION / OFFSET: 170+65, 86' RT.	EXPLORATION ID B-011-0-21															
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 885.9 (MSL) EOB: 7.5 ft.	PAGE															
START: 4/14/21 END: 4/14/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	LAT / LONG: 40.120386, -82.972642	1 OF 1															
MATERIAL DESCRIPTION AND NOTES	ELEV. 885.9	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				

18.0" ASPHALT AND 18.0" BASE (DRILLERS DESCRIPTION)
@1.5' TO 3.0'; SS-1 IS GRANULAR BASE

882.9	1																
	2	20															
	3	25	78	67	SS-1	-	-	-	-	-	-	-	-	-	5	A-1-b (V)	-
	4	32															
881.4	5	3	6	18	SS-2	4.50	8	11	14	37	30	26	16	10	12	A-4a (6)	440
	6	7	7														
	7	5	16	78	SS-3	4.50	14	11	13	35	27	28	17	11	13	A-6a (6)	-
878.4	8	6	6	18	SS-4	1.50	-	-	-	-	-	-	-	-	28	A-6a (V)	-
	9	7															
	EOB																

HARD, BROWNISH GRAY AND BROWN, **SANDY SILT**, SOME CLAY, TRACE GRAVEL, DAMP

STIFF TO HARD, BROWNISH GRAY AND BROWN, **SILT AND CLAY**, SOME SAND, LITTLE GRAVEL, DAMP TO WET

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 45B			STATION / OFFSET: 186+58, 84' RT.			EXPLORATION ID B-015-0-21										
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: I-71													
PID: 105435	SFN:	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 894.1 (MSL)			EOB:	7.5 ft.	PAGE								
START: 4/15/21	END: 4/15/21	SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.124754, -82.971942			1 OF 1										
MATERIAL DESCRIPTION AND NOTES			ELEV. 894.1	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL				
15.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)				892.3	1																
VERY DENSE, GRAY, STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, DAMP (FILL)					2	20	36	109	33	SS-1	-	-	-	-	-	-	12	A-1-b (V)	-		
VERY STIFF TO HARD, BROWNISH GRAY BECOMING GRAY, SANDY SILT, LITTLE TO SOME CLAY, LITTLE GRAVEL, DAMP					3	17	6	15	72	SS-2	4.50	15	17	19	31	18	28	18	10	2300	
					4	5															
					5	4	3	11	100	SS-3	3.25	18	14	14	32	22	25	16	9	11	A-4a (4)
					6	5															
					7	5	4	12	100	SS-4	3.25	-	-	-	-	-	-	-	13	A-4a (V)	-
				EOB																	
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																					
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																					

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 45B	STATION / OFFSET: 190+59, 84' RT.	EXPLORATION ID B-016-0-21																
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																	
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 896.6 (MSL) EOB: 7.5 ft.	PAGE																
START: 4/15/21 END: 4/15/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	LAT / LONG: 40.125854, -82.971865	1 OF 1																
MATERIAL DESCRIPTION AND NOTES	ELEV. 896.6	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
		GR	CS						FS	SI	CL	LL	PL	PI						
15.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)	894.8	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	X		
DENSE, BROWNISH GRAY, GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, RESEMBLES GRANULAR BASE, MOIST (FILL)	893.6	1	6	15 22	50	44	SS-1	-	-	-	-	-	-	-	-	14	A-1-b (V)	-		
HARD, BROWNISH GRAY AND GRAY, SANDY SILT, SOME CLAY, TRACE TO LITTLE GRAVEL, DAMP	889.1	2	6	6 7	18	100	SS-2	4.50	10	14	16	36	24	25	16	9	A-4a (5)	3767		
		3	6	10 8	25	100	SS-3	4.50	15	12	15	34	24	25	16	9	A-4a (5)	-		
		4	9	11 7	25	100	SS-4	4.50	-	-	-	-	-	-	-	-	13	A-4a (V)	-	
		5	7	EOB																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																				
ABANDONMENT METHODS. MATERIALS. QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH: SHOVELLED SOIL CUTTINGS																				

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 45B	STATION / OFFSET: 194+58, 96' RT.	EXPLORATION ID B-017-0-21															
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: I-71																
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 898.8 (MSL)	PAGE															
START: 4/15/21 END: 4/15/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	EOB: 7.5 ft.	1 OF 1															
MATERIAL DESCRIPTION AND NOTES	ELEV. 898.8	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				

14.5" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)		897.1		1																
HARD, BROWN AND BROWNISH GRAY, SILT AND CLAY, SOME SAND, LITTLE GRAVEL, DAMP		895.8		2	11	5	23	100	SS-1	4.50	15	12	16	33	24	32	19	13	A-6a (6)	193
HARD, BROWNISH GRAY, SANDY SILT, SOME CLAY, LITTLE GRAVEL, DAMP		894.3		3	9	9	29	100	SS-2	4.50	12	12	18	35	23	26	17	9	A-4a (5)	-
VERY STIFF TO HARD, GRAY, SILT AND CLAY, LITTLE SAND, LITTLE GRAVEL, DAMP		891.3	EOB	4	12	8	8	20	100	SS-3	4.50	-	-	-	-	-	-	11	A-6a (V)	-
				5	7	6	6	16	100	SS-4	3.75	-	-	-	-	-	-	11	A-6a (V)	-

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. LONG	DRILL RIG: CME 45B	STATION / OFFSET: 84+94, 3' LT.	EXPLORATION ID B-018-0-21
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. LONG	HAMMER: CME AUTOMATIC	ALIGNMENT: RAMP M	
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 12/5/19	ELEVATION: 921.6 (MSL) EOB: 7.5 ft.	PAGE
START: 4/19/21 END: 4/19/21	SAMPLING METHOD: SPT	ENERGY RATIO (%): 81.7	LAT / LONG: 40.110560, -82.989002	1 OF 1

MATERIAL DESCRIPTION AND NOTES	ELEV. 921.6	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL		
								GR	CS	FS	SI	CL	LL	PL	PI						
17.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)				1																	
		919.7		2	4			8	20	100	SS-1	4.50	24	13	8	29	26	36	20	16	
HARD, GRAY AND BROWN, SILTY CLAY, SOME STONE FRAGMENTS, SOME SAND, STONE FRAGMENTS ARE WEAK SHALE, DAMP STIFF TO VERY STIFF, BROWN WITH TRACE GRAY MOTTLES, CLAY, SOME SILT, SOME SAND, TRACE GRAVEL, MOIST TO DAMP		918.6		3	5			7	15	100	SS-2	2.25	5	6	16	33	40	50	22	28	
				4	5			6													
		914.1	EOB	5	3			4	10	100	SS-3	1.75	-	-	-	-	-	-	-	27	A-7-6 (V)
				6	4			4	12	100	SS-4	1.50	-	-	-	-	-	-	-	22	A-7-6 (V)
				7	4			5												-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS

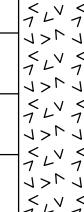
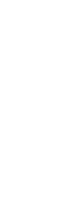
PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 92+48, 4' LT.			EXPLORATION ID B-020-0-21									
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP M												
PID: 105435 SFN:		DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 917.4 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1									
START: 4/19/21 END: 4/19/21		SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.110342, -82.986326												
MATERIAL DESCRIPTION AND NOTES			ELEV. 917.4	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL			
16.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)				915.6	1						GR	CS	FS	SI	CL	LL	PL	PI		
HARD, BROWN AND BROWNISH GRAY, SANDY SILT , SOME CLAY, TRACE GRAVEL, CONTAINS IRON STAINING, DAMP					2	15	10	29	100	SS-1	4.50	9	14	17	35	25	26	17	9	12
HARD, BROWN, SILT AND CLAY , SOME SAND, LITTLE GRAVEL, CONTAINS A 2.0" STONE FRAGMENT, DAMP					3	9	9	23	56	SS-2	4.50	12	14	15	34	25	27	16	11	12
VERY STIFF, GRAY, SILTY CLAY , LITTLE TO SOME SAND, LITTLE GRAVEL, DAMP					4	8														-
					5	3	3	10	44	SS-3	3.75	-	-	-	-	-	-	-	-	13
					6	4														-
					7	2	3	11	56	SS-4	2.25	-	-	-	-	-	-	-	-	14
						5														-
									</											

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 96+55, 5' RT.			EXPLORATION ID B-021-0-21															
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP M																		
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 918.6 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1															
START: 4/19/21	END: 4/19/21	SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.109798, -82.985064																		
MATERIAL DESCRIPTION AND NOTES			ELEV. 918.6	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL									
18.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)				916.6			1																			
HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP							2	8	SS-1	4.50	8	10	16	36	30	32	18	14	13	A-6a (8)	293					
							3	8	SS-2	4.50	9	9	17	37	28	29	18	11	14	A-6a (6)	-					
							4	7	SS-3	4.50	-	-	-	-	-	-	-	-	15	A-6a (V)	-					
							5	7	SS-4	4.50	-	-	-	-	-	-	-	-	15	A-6a (V)	-					
							6	8																		
							7	5																		
							EOB																			
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																										
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																										

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 103+72, 62' LT.			EXPLORATION ID B-023-0-21										
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP M													
PID: 105435	SFN:	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 923.6 (MSL)			EOB: 7.5 ft.	PAGE 1 OF 1									
START: 4/19/21	END: 4/19/21	SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.108675, -82.982948													
MATERIAL DESCRIPTION AND NOTES			ELEV. 923.6	DEPTH(S)	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL					
18.0" ASPHALT AND 5.0" BASE (DRILLERS DESCRIPTION)						1				GR	CS	FS	SI	CL	LL	PL	PI				
HARD, BROWN AND GRAY, SILTY CLAY, SOME SAND, LITTLE GRAVEL AND STONE FRAGMENTS, CONTAINS TRACE IRON STAINING, DAMP				921.7		6	7	SS-1	4.50	-	-	-	-	-	-	-	14	A-6b (V)	533		
VERY STIFF, GRAY AND BROWN, SILTY CLAY, "AND" STONE FRAGMENTS, SOME SAND, CONTAINS NO INTACT SOIL FOR HP READINGS, DAMP				917.6		7	8	SS-2	4.50	13	16	13	31	27	35	19	16	16	A-6b (7)	-	
				916.1	EOB	8	9	SS-3	4.25	-	-	-	-	-	-	-	-	11	A-6b (V)	-	
						9	16	SS-4	-	40	15	7	22	16	32	18	14	9	A-6a (2)	-	
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																					
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																					

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 1018+46, 4' RT.			EXPLORATION ID B-026-0-21							
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP P										
PID: 105435 SFN:		DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 928.1 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1							
START: 4/19/21 END: 4/19/21		SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.108377, -82.978429										
MATERIAL DESCRIPTION AND NOTES			ELEV. 928.1	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL	
16.0" ASPHALT AND 9.0" BASE (DRILLERS DESCRIPTION)				926.0	1													
VERY STIFF, BROWN AND BROWNISH GRAY, SILT AND CLAY, SOME SAND, TRACE TO LITTLE GRAVEL, DAMP					2	9	3	8	SS-1	3.00	6	9	16	37	32	32	17	A-6a (9) 673
					3	3	3	10	SS-2	3.75	6	9	14	38	33	33	19	A-6a (9) -
					4	4	3	4										
					5	3	2	5	SS-3	3.50	-	-	-	-	-	-	13	A-6a (V) -
					6	4	2	2										
					7	2	2	5	SS-4	2.50	-	-	-	-	-	-	17	A-6a (V) -
					EOB													
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																		
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																		

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 45B			STATION / OFFSET: 1022+29, 18' LT.			EXPLORATION ID B-027-0-21										
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP P													
PID: 105435 SFN:		DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 12/5/19			ELEVATION: 913.7 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1										
START: 4/19/21 END: 4/19/21		SAMPLING METHOD: SPT			ENERGY RATIO (%): 81.7			LAT / LONG: 40.109351, -82.977947													
MATERIAL DESCRIPTION AND NOTES			ELEV. 913.7	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL				
16.0" ASPHALT AND 8.0" BASE (DRILLERS DESCRIPTION)				911.7	1																
LOOSE, GRAY, STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, CONTAINS ASPHALT FRAGMENTS, DAMP (FILL)					2	6	3	7	SS-1	-	45	14	8	19	14	NP	NP	NP	10	A-2-4 (0)	-
STIFF, GRAY, SILT AND CLAY, SOME TO "AND" STONE FRAGMENTS, SOME SAND, CONTAINS NO INTACT SOIL FOR HP READINGS, DAMP					3	2															807
					4	3	7	19	SS-2	-	30	14	8	27	21	34	20	14	12	A-6a (4)	
					5	7	5	18	SS-3	-	-	-	-	-	-	-	-	-	16	A-6a (V)	-
					6	6	8														
					7	6	6	18	SS-4	-	-	-	-	-	-	-	-	-	14	A-6a (V)	-
					EOB																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																					
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVELED SOIL CUTTINGS																					

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 75T			STATION / OFFSET: 13+63, 23' RT.			EXPLORATION ID B-028-0-21													
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP K																
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 921.3 (MSL)			EOB: 7.5 ft.	PAGE 1 OF 1												
START: 8/8/22	END: 8/8/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 79			LAT / LONG: 40.110243, -82.972635																
MATERIAL DESCRIPTION AND NOTES			ELEV. 921.3	DEPTH(S)	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL								
9.0" ASPHALT AND 9.0" CONCRETE AND 5.0" BASE (DRILLERS DESCRIPTION)				919.4		1				GR	CS	FS	SI	CL	LL	PL	PI							
HARD, BROWN, SILTY CLAY, SOME SAND, TRACE GRAVEL, DAMP						2	7	5	12	56	SS-1	4.50	9	12	16	34	29	34	17	17	15	A-6b (8)	200	
						3	4	4	13	100	SS-2	4.50	5	7	17	36	35	34	18	16	16	A-6b (10)	-	
						4	6														15	A-6b (V)	-	
						5	2	5	13	100	SS-3	4.50	-	-	-	-	-	-	-	-	11	A-6b (V)	-	
						6	3	5	16	100	SS-4	4.50	-	-	-	-	-	-	-	-				
						7	7																	
				EOB																				
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																								
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																								

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 75T			STATION / OFFSET: 16+67, 25' RT.			EXPLORATION ID B-029-0-21												
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP K															
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 929.0 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1												
START: 8/8/22	END: 8/8/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 79			LAT / LONG: 40.110499, -82.973656															
MATERIAL DESCRIPTION AND NOTES			ELEV. 929.0	DEPTH(S)	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL							
8.0" ASPHALT AND 9.0" CONCRETE AND 5.0" BASE (DRILLERS DESCRIPTION)				927.2		1				GR	CS	FS	SI	CL	LL	PL	PI						
HARD, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP				926.0		2	3	9	44	SS-1	4.50	4	8	16	39	33	32	18	14	17	A-6a (9)	280	
VERY STIFF, BROWN, SILTY CLAY, LITTLE TO SOME SAND, TRACE GRAVEL, MOIST				921.5		3	4			SS-2	2.25	7	7	16	36	34	35	18	17	20	A-6b (10)	-	
				EOB		4	5	16	50	SS-3	2.75	-	-	-	-	-	-	-	-	21	A-6b (V)	-	
						5	2	9	100	SS-4	3.00	-	-	-	-	-	-	-	-	22	A-6b (V)	-	
						6	3	4															
						7	5	14	100														
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																							
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																							

PID: 105435	SFN: 2511460	PROJECT: FRA-071/270-28.27/25.99A	STATION / OFFSET: 19+68, 27' RT.	START: 8/9/22	END: 8/9/22	PG 2 OF 2	B-030-0-21												
MATERIAL DESCRIPTION AND NOTES	ELEV. 907.1	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				
HARD, BROWN, SILTY CLAY, LITTLE SAND, TRACE GRAVEL, DAMP (continued)				3	6	SS-14	4.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	
		31	11	22	56														
		32																	
		33	3	5	17	SS-15	4.25	-	-	-	-	-	-	-	-	15	A-6b (V)	-	
		34	8																
		35	5	14	100	SS-16	4.50	-	-	-	-	-	-	-	-	14	A-6b (V)	-	
		36	5	6	100														
		37																	
		38																	
		39	4	5	13	SS-17	4.50	-	-	-	-	-	-	-	-	15	A-6b (V)	-	
		40	5																
	897.1	EOB																	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; PUMPED 50 GAL. BENTONITE GROUT; SHOVED SOIL CUTTINGS

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; PUMPED 50 GAL. BENTONITE GROUT; SHOVELED SOIL CUTTINGS

PID: 105435 SFN: 2511372 PROJECT: FRA-071/270-28.27/25.99A STATION / OFFSET: 26+46, 10' RT. START: 8/1/22 END: 8/2/22 PG 3 OF 4 B-033-0-21B

NOTES: GROUNDWATER ENCOUNTERED AT 18.0' DURING DRILLING. HOLE DID NOT CAVE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; PUMPED 250 GAL. BENTONITE GROUT

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 75T			STATION / OFFSET: 30+16, 2' LT.			EXPLORATION ID B-034-0-21							
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP K										
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 926.7 (MSL) EOB: 11.5 ft.			PAGE 1 OF 1							
START: 8/10/22	END: 8/10/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 79			LAT / LONG: 40.112095, -82.977907										
MATERIAL DESCRIPTION AND NOTES			ELEV. 926.7	DEPTH(S)	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL		
8.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)			925.5			1				GR	CS	FS	SI	CL	LL	PL	PI	
HARD, BROWN, SILTY CLAY, LITTLE TO SOME SAND, TRACE GRAVEL, IRON STAINING, DAMP						2												
						3	4	SS-1	4.50	3	7	14	38	38	37	18	19	17 A-6b (12) -
						4	5											
						5	4	SS-2	4.50	-	-	-	-	-	-	-	-	13 A-6b (V) -
HARD, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP TO MOIST			919.7			6	8											
						7												
						8	9	SS-3	4.50	6	10	15	36	33	32	18	14	15 A-6a (8) -
						9	10											
						10	7	SS-4	4.50	-	-	-	-	-	-	-	-	20 A-6a (V) -
						11	8											
				EOB														
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																		
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																		

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. HODGES			DRILL RIG: CME 75T			STATION / OFFSET: 34+09, 22' RT.			EXPLORATION ID B-035-0-22											
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. HODGES			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP K														
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 915.6 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1											
START: 8/10/22	END: 8/10/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 79			LAT / LONG: 40.111794, -82.979258														
MATERIAL DESCRIPTION AND NOTES			ELEV. 915.6	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL					
9.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)			914.3				1															
HARD, BROWN, SILTY CLAY, SOME SAND, TRACE GRAVEL, DAMP							2	5	SS-1	4.50	-	-	-	-	-	-	13	A-6b (V)	-			
							3	7														
							4	8	SS-2	4.50	6	8	16	35	35	34	17	17	14	A-6b (10)	80	
							5	11														
							6	9	SS-3	4.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	
							7	12														
HARD, BROWNISH GRAY AND DARK GRAY, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, SLIGHTLY ORGANIC (3.1% LOI), CONTAINS A 1.5" SILT SEAM, DAMP			909.6				8	10	SS-4	4.50	2	4	10	47	37	34	22	12	19	A-6a (9)	-	
							9	11														
							10															
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																						
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																						

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; PUMPED 250 GAL. BENTONITE GROUT

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. HODGES	DRILL RIG: CME 75T	STATION / OFFSET: 31+80, 5' LT.	EXPLORATION ID B-039-0-21																	
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. HODGES	HAMMER: CME AUTOMATIC	ALIGNMENT: RAMP O																		
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 1/24/22	ELEVATION: 929.8 (MSL)	PAGE																	
START: 8/10/22 END: 8/10/22	SAMPLING METHOD: SPT	ENERGY RATIO (%): 79	EOB: 7.5 ft.	1 OF 1																	
MATERIAL DESCRIPTION AND NOTES		ELEV. 929.8	DEPTH(S)		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
8.0" ASPHALT AND 7.0" BASE (DRILLERS DESCRIPTION)		928.5	-	-	-	-	-	-	-	GR	CS	FS	SI	CL	LL	PL	PI				
HARD, BROWN AND BROWNISH GRAY BECOMING GRAY, SILT AND CLAY, SOME SAND, TRACE TO LITTLE GRAVEL, DAMP		922.3	-	-	-	1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
		EOB	8	6	18	39	SS-1	4.50	8	13	17	34	28	27	16	11	12	A-6a (6)	400		
			5	7	16	100	SS-2	4.50	-	-	-	-	-	-	-	-	12	A-6a (V)	-		
			5	6	14	100	SS-3	4.50	-	-	-	-	-	-	-	-	13	A-6a (V)	-		
			5	7	16	100	SS-4	4.50	14	10	14	35	27	27	14	13	11	A-6a (7)	-		

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 55T 22			STATION / OFFSET: 37+47, 58' LT.			EXPLORATION ID B-041-0-21									
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP O												
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 921.7 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1									
START: 7/26/22	END: 7/26/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 63.4			LAT / LONG: 40.111728, -82.979914												
MATERIAL DESCRIPTION AND NOTES			ELEV. 921.7	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL			
4.0" ASPHALT AND 8.0" CONCRETE AND 6.0" BASE (DRILLERS DESCRIPTION)				920.2	1															
HARD, GRAY, SANDY SILT, SOME CLAY, TRACE GRAVEL, DAMP					2	4	11	33	SS-1	4.50	9	11	15	38	27	25	15	10		
HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP					3	5	6												2633	
					4	5	13	19	SS-2	4.50	5	10	16	36	33	32	17	15		
					5	8	6	19	SS-3	4.50	-	-	-	-	-	-	-	-		
					6	6	12													
					7	6	11	27	SS-4	4.50	-	-	-	-	-	-	-	-		
					EOB															
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																				
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																				

PROJECT: FRA-071/270-28.27/25.99A		DRILLING FIRM / OPERATOR: NEAS / J. LONG			DRILL RIG: CME 55T 22			STATION / OFFSET: 141+47, 31' RT.			EXPLORATION ID B-042-0-21										
TYPE: SUBGRADE		SAMPLING FIRM / LOGGER: NEAS / J. LONG			HAMMER: CME AUTOMATIC			ALIGNMENT: RAMP N													
PID: 105435	SFN: _____	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 910.3 (MSL) EOB: 7.5 ft.			PAGE 1 OF 1										
START: 7/26/22	END: 7/26/22	SAMPLING METHOD: SPT			ENERGY RATIO (%): 63.4			LAT / LONG: 40.112634, -82.975132													
MATERIAL DESCRIPTION AND NOTES			ELEV. 910.3	DEPTHs		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL				
12.0" ASPHALT AND 6.0" BASE (DRILLERS DESCRIPTION)			908.8		1																
MEDIUM DENSE, GRAY, STONE FRAGMENTS WITH SAND AND SILT, TRACE CLAY, GRANULAR BASE, DAMP			907.3		2	26	11	20	SS-1	-	-	-	-	-	-	7	A-2-4 (V)	-			
VERY STIFF TO HARD, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP TO MOIST			904.3		3	6	7	16	SS-2	4.50	7	11	17	37	28	30	17	13	14	A-6a (7)	1280
VERY STIFF, DARK BROWN, CLAY, "AND" SILT, TRACE SAND, TRACE GRAVEL, MODERATELY ORGANIC (5.0% LOI), CONTAINS A STRONG PETROLIFEROUS ODOR, MOIST			902.8		4	4	3	8	SS-3	2.75	-	-	-	-	-	-	-	22	A-6a (V)	-	
			EOB		5	5	7	18	SS-4	3.50	2	2	7	40	49	53	25	28	28	A-7-6 (18)	-
					6	7	10														
					7																
NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.																					
ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS																					

PROJECT: FRA-071/270-28.27/25.99A	DRILLING FIRM / OPERATOR: NEAS / J. LONG	DRILL RIG: CME 55T 22	STATION / OFFSET: 145+00, 11' RT.	EXPLORATION ID B-043-0-21															
TYPE: SUBGRADE	SAMPLING FIRM / LOGGER: NEAS / J. LONG	HAMMER: CME AUTOMATIC	ALIGNMENT: RAMP N																
PID: 105435 SFN:	DRILLING METHOD: 3.25" HSA	CALIBRATION DATE: 1/24/22	ELEVATION: 906.0 (MSL) EOB: 7.5 ft.	PAGE															
START: 7/26/22 END: 7/26/22	SAMPLING METHOD: SPT	ENERGY RATIO (%): 63.4	LAT / LONG: 40.113588, -82.975142	1 OF 1															
MATERIAL DESCRIPTION AND NOTES	ELEV. 906.0	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
								GR	CS	FS	SI	CL	LL	PL	PI				

12.0" ASPHALT AND 8.0" BASE (DRILLERS DESCRIPTION)		904.3		1														
				2	22													
				3	13	27	67	SS-1	4.50	6	10	18	39	27	28	17	11	14
				4	11	22	94	SS-2	4.50	-	-	-	-	-	-	-	-	13
				5	10	11												
				6	5	8	17	SS-3	4.50	16	14	17	32	21	25	18	7	12
				7	6	8												
HARD, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP		901.5																
HARD, GRAY AND ORANGISH BROWN, SANDY SILT, SOME CLAY, LITTLE GRAVEL, IRON STAINING, DAMP		898.5	EOB															

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVED SOIL CUTTINGS

PID:	SFN:	PROJECT:FRA-71/270-28.27/25.99A MOD2	STATION / OFFSET:	168+80, 82' RT.	START:	10/25/23	END:	10/26/23	PG 2 OF 2	B-044-0-22										
MATERIAL DESCRIPTION AND NOTES			ELEV. 856.6	DEPTHs	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)					ATTERBERG			WC	ODOT CLASS (GI)	BACK FILL
										GR	CS	FS	SI	CL	LL	PL	PI			
VERY STIFF TO HARD, GRAY, SILT AND CLAY , TRACE TO SOME SAND, TRACE TO LITTLE GRAVEL, GLACIAL TILL, SS-8 CONTAINS NO INTACT SOIL FOR HP READINGS, DAMP <i>(continued)</i>				- 31 32 33 34 35 36	5 7 8 6 6 9 6 7 10	16 16 16 18	94 89 100	SS-12 SS-13 SS-14	4.25 3.50 3.50	- - - - - - - - -	- - - - - - - - -	- - - - - - - - -	- - - - - - - - -	- - - - - - - - -	16 16 15	A-6a (V) A-6a (V) A-6a (V)				
			850.1	EOB																

NOTES: GROUNDWATER ENCOUNTERED AT 17.0' DURING DRILLING. HOLE DID NOT CAVE. DRILLED AS STAKED.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; Poured 0.5 BAG HOLE PLUG; SHOVED SOIL CUTTINGS

PROJECT: FRA-71/270-28.27/25.99A MOD2		DRILLING FIRM / OPERATOR: NEAS / ASHBAUGH			DRILL RIG: CME 55T			STATION / OFFSET: 172+60, 80' RT.			EXPLORATION ID: B-046-0-22				
TYPE: RETAINING WALL		SAMPLING FIRM / LOGGER: NEAS / ASHBAUGH			HAMMER: CME AUTOMATIC			ALIGNMENT: IR-71							
PID:	SFN:	DRILLING METHOD: 3.25" HSA			CALIBRATION DATE: 1/24/22			ELEVATION: 886.3 (MSL) EOB: 36.5 ft.			PAGE 1 OF 2				
START:	END:	SAMPLING METHOD: SPT			ENERGY RATIO (%): 63.4			LAT / LONG: 40.120920, -82.972536							
MATERIAL DESCRIPTION AND NOTES			ELEV. 886.3	DEPTH(S)	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GRADATION (%)		ATTERBERG	WC	ODOT CLASS (GI)	BACK FILL
8.0" ASPHALT AND 5.0" CONCRETE (DRILLERS DESCRIPTION)				885.2											
MEDIUM DENSE, GRAY, STONE FRAGMENTS , SOME SAND, LITTLE SILT, TRACE CLAY, GRANULAR BASE, DAMP (FILL)				882.3											
VERY STIFF, BROWN AND DARK GRAY, SILT AND CLAY , SOME SAND, TRACE TO LITTLE GRAVEL, CONTAINS PLASTIC TRASH FRAGMENTS, DAMP (FILL)				871.8											
STIFF TO VERY STIFF, BROWN AND GRAY, SILT AND CLAY , LITTLE SAND, TRACE TO LITTLE GRAVEL, DAMP TO MOIST				861.8											
VERY STIFF TO HARD, GRAY, SILT AND CLAY , LITTLE SAND, TRACE GRAVEL, GLACIAL TILL, DAMP TO MOIST															

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE. DRILLED AS STAKED.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; POURED 0.5 BAG HOLE PLUG; SHOVELED SOIL CUTTINGS

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE. DRILLED AS STAKED.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; POURED 0.5 BAG HOLE PLUG; SHOVED SOIL CUTTINGS



OHIO DEPARTMENT OF TRANSPORTATION

DETERMINING SULFATE CONTENT IN SOILS

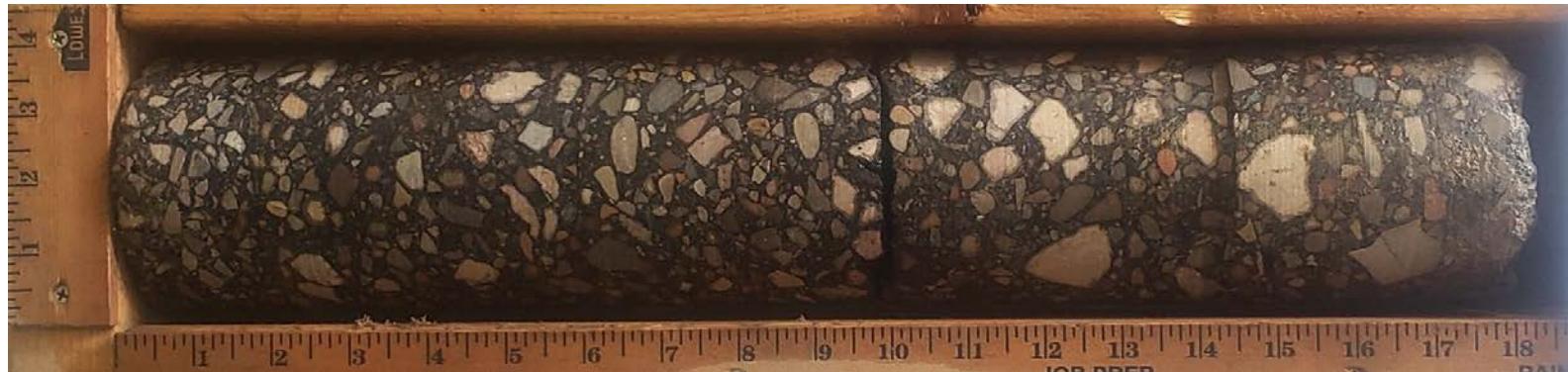
SUPPLEMENT 1122

Project C-R-S: FRA-071/270-28.27
PID No: 105435
Report Date: 9/1/2022
Consultant: NEAS Inc.
Technician: L. Rosenbeck

Boring ID & Sample #	Station	Offset	Latitude & Longitude or State Plane Coordinates	Elevation	Soaking Time (hr)	Replicate Sample Readings						Sulfate Content (ppm)	
						1		2		3			
						Dilution	Reading	Dilution	Reading	Dilution	Reading		
B-001-0-21 SS-2	130+13.45	33.64' RT.	40.110037	-82.977651	913.02	17.1	20	11	20	12	20	12	233
B-002-0-21 SS-1	134+38.61	48.74' RT.	40.111052	-82.976900	914.113	17.1	20	19	20	21	20	20	400
B-003-0-21 SS-3	138+27.06	46.29' RT	40.112010	-82.976279	913.661	17.1	20	5	20	5	20	5	100
B-004-0-21 SS-2	142+23.72	49.14' RT	40.112998	-82.975669	911.111	17.1	20	1	20	1	20	1	20
B-005-0-21 SS-2	146+00.36	27.13' RT	40.113975	-82.975211	908.243	17.1	20	12	20	14	20	13	260
B-006-0-21 SS-1	150+63.61	59.05' RT	40.115133	-82.974503	902.988	18.1	20	32	20	29	20	26	580
B-007-0-21 SS-1	154+37.05	56.17' RT	40.116108	-82.974071	899.363	18.2	20	52	20	58	20	62	1147
B-008-0-21 SS-1	158+40.65	50.43' RT	40.117174	-82.973656	895.22	18.1	100	46	100	42	100	40	4267
B-009-0-21 SS-1	162+45.49	55.74' RT	40.118246	-82.973248	890.984	16.3	100	>80	100	>80	100	>80	>8000
B-010-0-21 SS-2	166+30.55	49.53' RT	40.119281	-82.972942	887.8	16.3	100	61	100	56	100	56	5767
B-011-0-21 SS-2	170+40.03	48.88' RT	40.120386	-82.972642	885.897	16.3	20	22	20	22	20	22	440
B-012-0-21 SS-2	174+32.33	48.74' RT	40.121451	-82.972399	886.806	16.3	20	9	20	14	20	10	220
B-013-0-21 SS-2	178+30.59	47.78' RT	40.122537	-82.972200	889.171	16.3	100	14	100	15	100	14	1433
B-014-0-21 SS-1	182+39.72	46.26' RT	40.123658	-82.972045	891.613	21.3	100	53	100	45	100	45	4767
B-015-0-21 SS-2	186+38.44	44.65' RT	40.124754	-82.971942	894.132	21.3	100	27	100	22	100	20	2300
B-016-0-21 SS-2	190+39.61	43.68' RT	40.125854	-82.971865	896.565	21.3	100	39	100	35	100	39	3767
B-017-0-21 SS-1	194+38.28	56.43' RT	40.126945	-82.971739	898.808	21.3	20	11	20	10	20	8	193
B-018-0-21 SS-1	84+54.58	16.59' RT	40.110560	-82.989002	921.592	21.3	20	10	20	11	20	11	213
B-019-0-21 SS-1	88+33.68	26.22' LT	40.110619	-82.987640	920.357	21.3	20	28	20	32	20	35	633
B-020-0-21 SS-1	92+14.25	24.92' RT	40.110342	-82.986326	917.375	16.4	20	19	20	20	20	20	393
B-021-0-21 SS-1	96+33.42	43.71' RT	40.109798	-82.985064	918.563	16.4	20	14	20	15	20	15	293

B-022-0-21 SS-3	99+80.90	42.93' RT	40.109199	-82.984109	920.637	16.4	20	12	20	12	20	12	240
B-023-0-21 SS-1	103+52.16	22.33' LT	40.108675	-82.982948	923.572	16.4	20	26	20	27	20	27	533
B-024-0-21 SS-2	107+35.10	9.60' LT	40.107974	-82.981926	925.586	16.3	100	23	100	16	100	20	1967
B-025-0-21 SS-2	1010+98.56	CL	40.107701	-82.980661	935.545	16.4	40	30	40	31	40	31	1227
B-026-0-21 SS-1	1018+05.87	16.63' LT	40.108377	-82.978429	928.067	16.4	20	35	20	33	20	33	673
B-027-0-21 SS-2	1022+01.45	52.46' LT	40.109351	-82.977947	913.74	16.4	20	38	20	46	20	37	807
B-028-0-21 SS-1						18.0	20	10	20	10	20	10	200
B-029-0-21 SS-1						18.0	20	14	20	14	20	14	280
B-032-0-21 SS-1						18.0	20	12	20	12	20	12	240
B-035-0-21 SS-2						18.0	20	4	20	4	20	4	80
B-036-0-21 SS-1						18.0	20	30	20	30	20	30	600
B-037-0-21 SS-1						19.5	20	43	20	38	20	36	780
B-039-0-21 SS-1						19.5	20	21	20	20	20	19	400
B-040-0-21 SS-1						19.5	100	64	100	60	100	57	6033
B-041-0-21 SS-1						19.5	100	24	100	25	100	30	2633
B-042-0-21 SS-2						18.0	40	33	40	32	40	31	1280
B-043-0-21 SS-1						18.0	20	8	20	8	20	8	160

Core Photo: X-001-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		18.75		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	10			
2	8.75			
3				
4				
Rebar Encountered	N/A			

Latitude: 40.107653
Longitude: -82.979486

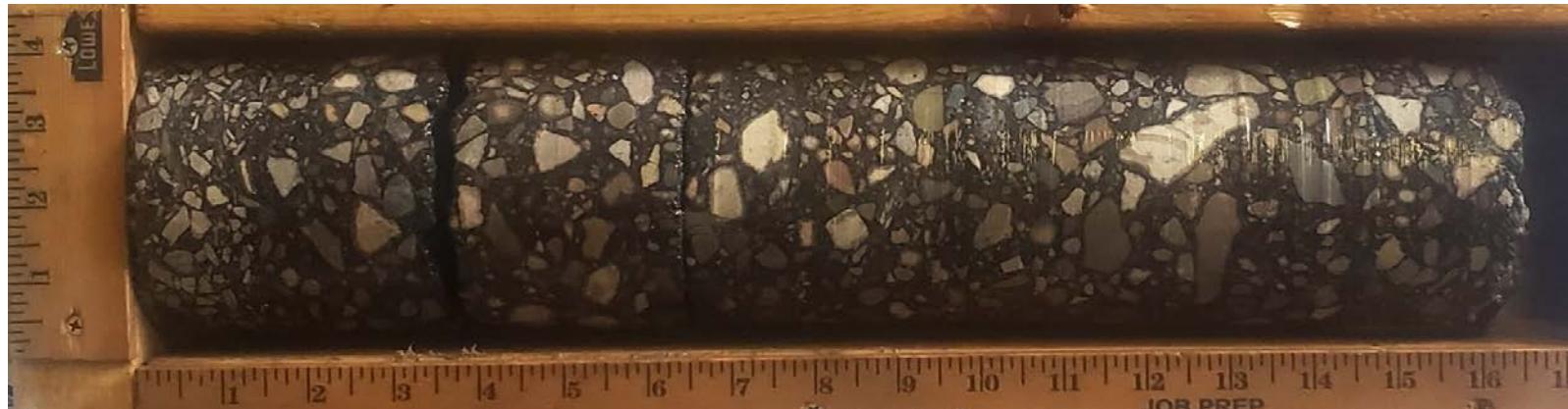
Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-002-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		16.75		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	3.5			
2	3			
3	10.25			
4				
Rebar Encountered	N/A			

Latitude: 40.113628
Longitude: -82.975595

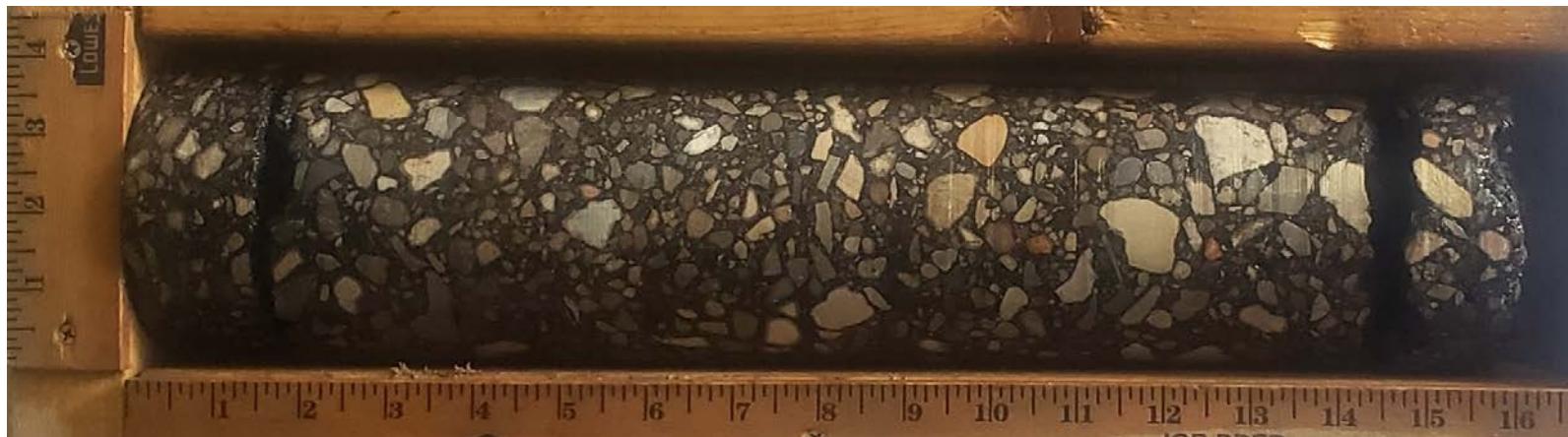
Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-003-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		16.5		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	1.5			
2	15			
3				
4				
Rebar Encountered	N/A			

Latitude: 40.120010
Longitude: -82.973021

Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-004-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		17		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	3.5			
2	5			
3	8.5			
4				
Rebar Encountered	N/A			

Latitude: 40.125285
Longitude: -82.972180

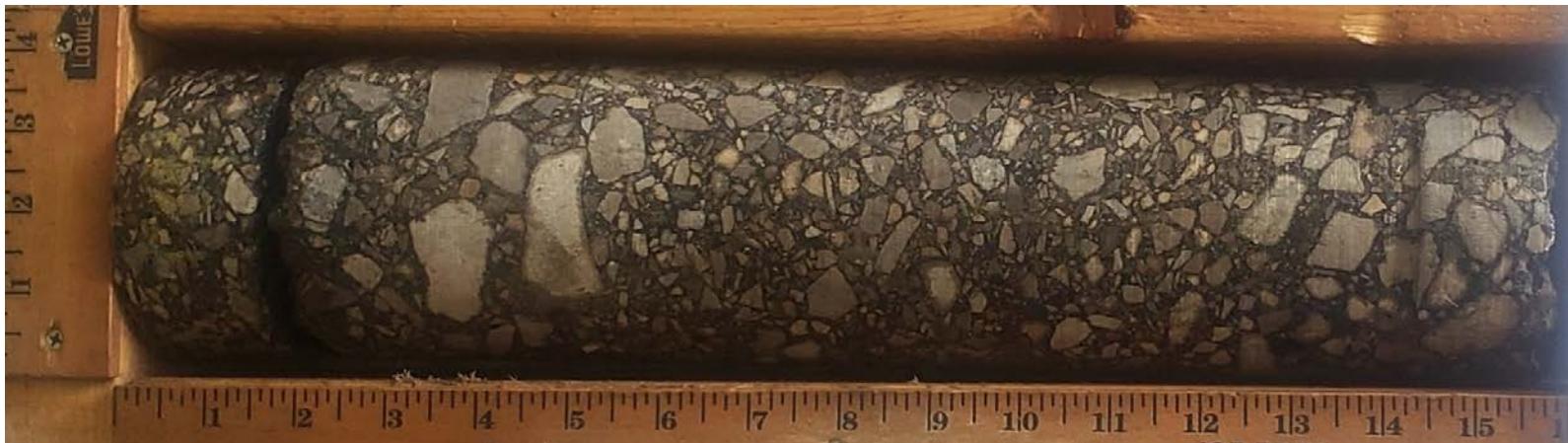
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Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-005-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		16.5		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	1.5			
2	15			
3				
4				
Rebar Encountered	N/A			

Latitude: 40.111052
Longitude: -82.976900

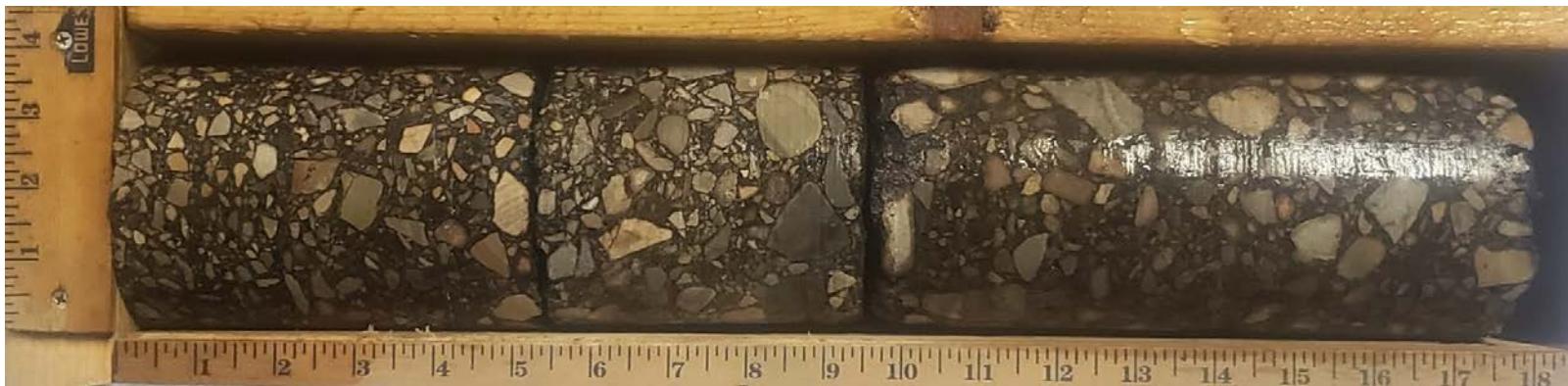
Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-006-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		18		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	2			
2	3.25			
3	4.25			
4	8.5			
Rebar Encountered	N/A			

Latitude: 40.118246
Longitude: -82.973248

Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

Core Photo: X-007-0-21



Core Information				
Core Diameter (in):		4		
Core Total Length (in):		14.75		
Layers	Core Composition & Thickness (in)			Remarks
	Asphalt	Concrete	Brick	
1	3.5			
2	4.5			
3	6.75			
4				
Rebar Encountered	N/A			

Latitude: 40.125854
Longitude: -82.971865

Pavement & Core Photo Log



Roadway Project
FRA-71/270

NEAS Project No.: 105435
Date: 4/21/2021
Taken By: MJ
Scale: N/A

APPENDIX C

GEOTECHNICAL BULLETIN 1 (GB1) ANALYSIS SPREADSHEETS

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****Entire Project - Improve Capacity to IR-270 WB to IR-71 NB****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 7, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**43**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-001-0-21	I-71	130+48	62	RT	CME 45B	82	913.0	910.8	2.2 C
2	B-002-0-21	I-71	134+73	77	RT	CME 45B	82	914.1	912.2	1.9 C
3	B-003-0-21	I-71	138+60	75	RT	CME 45B	82	913.7	912.0	1.7 C
4	B-004-0-21	I-71	142+56	79	RT	CME 45B	82	911.1	909.7	1.4 C
5	B-005-0-21	I-71	146+32	58	RT	CME 45B	82	908.2	906.0	2.2 C
6	B-006-0-21	I-71	150+94	91	RT	CME 45B	82	903.0	901.4	1.6 C
7	B-007-0-21	I-71	154+66	89	RT	CME 45B	82	899.4	897.7	1.7 C
8	B-008-0-21	I-71	158+69	85	RT	CME 45B	82	895.2	893.6	1.6 C
9	B-009-0-21	I-71	162+72	91	RT	CME 45B	82	891.0	889.6	1.4 C
10	B-010-0-21	I-71	166+56	86	RT	CME 45B	82	887.8	885.9	1.9 C
11	B-011-0-21	I-71	170+65	86	RT	CME 45B	82	885.9	884.7	1.2 C
12	B-012-0-21	I-71	174+56	86	RT	CME 45B	82	886.8	886.3	0.5 C
13	B-013-0-21	I-71	178+53	86	RT	CME 45B	82	889.2	888.0	1.2 C
14	B-014-0-21	I-71	182+61	85	RT	CME 45B	82	891.6	889.7	1.9 C
15	B-015-0-21	I-71	186+58	84	RT	CME 45B	82	894.1	891.3	2.8 C
16	B-016-0-21	I-71	190+59	84	RT	CME 45B	82	896.6	893.0	3.6 C
17	B-017-0-21	I-71	194+58	96	RT	CME 45B	82	898.8	894.7	4.2 C
18	B-018-0-21	Ramp M	84+94	3	LT	CME 45B	82	921.6	920.5	1.1 C
19	B-019-0-21	Ramp M	88+73	46	LT	CME 45B	82	920.4	918.1	2.2 C
20	B-020-0-21	Ramp M	92+48	4	LT	CME 45B	82	917.4	915.373	2.0 C
21	B-021-0-21	Ramp M	96+55	5	RT	CME 45B	82	918.6	916.6	1.9 C
22	B-022-0-21	Ramp M	100+01	3	RT	CME 45B	82	920.6	919.4	1.3 C
23	B-023-0-21	Ramp M	103+72	62	LT	CME 45B	82	923.6	922.4	1.2 C
24	B-024-0-21	Ramp M	107+55	49	LT	CME 45B	82	925.6	924.3	1.3 C
25	B-025-0-21	Ramp P	1011+38	22	LT	CME 45B	82	935.5	935.2	0.3 C
26	B-026-0-21	Ramp P	1018+46	4	RT	CME 45B	82	928.1	926.6	1.4 C
27	B-027-0-21	Ramp P	1022+29	18	LT	CME 45B	82	913.7	911.1	2.6 C
28	B-028-0-21	Ramp K	13+63	23	RT	CME 75T	79	921.3	922.8	1.6 F
29	B-029-0-21	Ramp K	16+67	25	RT	CME 75T	79	929.0	928.9	0.1 C
30	B-030-0-21	Ramp K	19+68	27	RT	CME 75T	79	937.1	938.6	1.5 F
31	B-031-0-21	Ramp K	21+80	11	LT	CME 75T	79	940.6	941.9	1.3 F
32	B-032-0-21	Ramp K	24+63	1	RT	CME 75T	79	938.9	939.2	0.3 F
33	B-033-0-21	I-71	137+81	9	RT	CME 55T/CME 75T	68.4/79	913.3	912.2	1.0 C
34	B-034-0-21	Ramp K	30+16	2	LT	CME 75T	79	926.7	925.8	0.8 C
35	B-035-0-21	Ramp K	34+09	22	RT	CME 75T	79	915.6	913.2	2.5 C
36	B-036-0-21	Ramp O	19+28	29	RT	CME 75T	79	916.2	914.5	1.7 C
37	B-037-0-21	Ramp O	23+07	2	RT	CME 75T	79	926.4	925.7	0.7 C
38	B-038-0-21	I-71	139+20	11	RT	CME 75T	79	912.8	911.7	1.1 C
39	B-039-0-21	Ramp O	31+80	5	LT	CME 75T	79	929.8	928.8	1.0 C
40	B-040-0-21	Ramp O	34+43	20	RT	CME 55T	63	925.8	924.7	1.1 C
41	B-041-0-21	Ramp O	37+47	58	LT	CME 55T	63	921.7	921.3	0.4 C
42	B-042-0-21	Ramp N	141+47	31	RT	CME 55T	63	910.3	909.6	0.7 C
43	B-043-0-21	Ramp N	145+00	11	RT	CME 55T	63	906.0	906.2	0.2 F



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
1	B 001-0 21	SS-1	1.5	3.0	-0.7	0.8	19	19	4.5	NP	NP	NP	24	15	39	10	11	A-4a	1	233							
		SS-2	3.0	4.5	0.8	2.3	20			39	19	20	43	41	84	21	16	A-6b	12			Mc					
		SS-3	4.5	6.0	2.3	3.8	29			4.5									A-6b	16							
		SS-4	6.0	7.5	3.8	5.3	26			3.75									A-6b	16							
2	B 002-0 21	SS-1	1.5	3.0	-0.4	1.1	27	11	4.5	27	17	10	39	27	66	13	12	A-4a	6	400							
		SS-2	3.0	4.5	1.1	2.6	19			28	17	11	35	22	57	11	14	A-6a	5								
		SS-3	4.5	6.0	2.6	4.1	15			4.5									19	14	A-6a	10					
		SS-4	6.0	7.5	4.1	5.6	11			2.75									21	18	A-7-6	16					
3	B 003-0 21	SS-1	1.5	3.0	-0.2	1.3	35	18	2.75									9	6	A-1-a	0						
		SS-2	3.0	4.5	1.3	2.8	30			NP	NP	NP	6	2	8	4	6	A-1-a	0								
		SS-3	4.5	6.0	2.8	4.3	18			32	18	14	34	29	63	18	14	A-6a	7	100							
		SS-4	6.0	7.5	4.3	5.8	22			2.25								21	14	A-6a	10						
4	B 004-0 21	SS-1	1.5	3.0	0.1	1.6	60	25	4.5									9	6	A-1-b	0						
		SS-2	3.0	4.5	1.6	3.1	35			34	19	15	40	33	73	17	14	A-6a	10	20							
		SS-3	4.5	6.0	3.1	4.6	29			32	17	15	36	32	68	16	14	A-6a	9								
		SS-4	6.0	7.5	4.6	6.1	25			3.5								24	18	A-7-6	16						
5	B 005-0 21	SS-1	1.5	3.0	-0.7	0.8	35	20	4.25									8	6	A-1-b	0						
		SS-2	3.0	4.5	0.8	2.3	23			26	17	9	34	22	56	14	12	A-4a	4	260							
		SS-3	4.5	6.0	2.3	3.8	20			4								15	10	A-4a	8						
		SS-4	6.0	7.5	3.8	5.3	23											12	10	A-4a	8						
6	B 006-0 21	SS-1	1.5	3.0	-0.1	1.4	18	18	4.5	24	16	8	35	24	59	13	11	A-4a	5	580							
		SS-2	3.0	4.5	1.4	2.9	30			25	16	9	27	17	44	9	11	A-4a	2								
		SS-3	4.5	6.0	2.9	4.4	37											9	6	A-1-b	0						
		SS-4	6.0	7.5	4.4	5.9	41			4.5								10	10	A-4a	8						
7	B 007-0 21	SS-1	1.5	3.0	-0.2	1.3	27	10	4.25									5	6	A-1-b	0	1147					
		SS-2	3.0	4.5	1.3	2.8	20			23	16	7	27	15	42	9	11	A-4a	1								
		SS-3	4.5	6.0	2.8	4.3	11			23	15	8	31	19	50	11	10	A-4a	3								
		SS-4	6.0	7.5	4.3	5.8	10			2.25								12	10	A-4a	8						
8	B 008-0 21	SS-1	1.5	3.0	-0.1	1.4	41	19	4.25	23	16	7	34	22	56	9	11	A-4a	4	4267							
		SS-2	3.0	4.5	1.4	2.9	19			23	15	8	36	22	58	10	10	A-4a	5								
		SS-3	4.5	6.0	2.9	4.4	19			4								12	10	A-4a	8						
		SS-4	6.0	7.5	4.4	5.9	22			4.25								11	10	A-4a	8						
9	B 009-0 21	SS-1	1.5	3.0	0.1	1.6	33	12	4.5									9	10	A-4a	8	>8000					
		SS-2	3.0	4.5	1.6	3.1	25			24	14	10	37	23	60	9	10	A-4a	5								
		SS-3	4.5	6.0	3.1	4.6	12			22	15	7	37	21	58	11	10	A-4a	5								
		SS-4	6.0	7.5	4.6	6.1	15			1.75								13	10	A-4a	8						



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
10	B 010-0 21	SS-1	1.5	3.0	-0.4	1.1	52	20	4.5							5	6	A-1-a	0	5767							
		SS-2	3.0	4.5	1.1	2.6	20			33	25	8	35	14	49	21	20	A-4a	3								
		SS-3	4.5	6.0	2.6	4.1	27			4.5							12	10	A-4a	8							
		SS-4	6.0	7.5	4.1	5.6	26			3.5	33	19	14	43	31	74	20	14	A-6a	9							
11	B 011-0 21	SS-1	1.5	3.0	0.3	1.8	78	16	4.5								5	6	A-1-b	0	440						
		SS-2	3.0	4.5	1.8	3.3	18			26	16	10	37	30	67	12	11	A-4a	6								
		SS-3	4.5	6.0	3.3	4.8	16			28	17	11	35	27	62	13	14	A-6a	6								
		SS-4	6.0	7.5	4.8	6.3	18			1.5							28	14	A-6a	10							
12	B 012-0 21	SS-1	1.5	3.0	1.0	2.5	52	14	4.5								10	6	A-1-b	0	220						
		SS-2	3.0	4.5	2.5	4.0	15			28	17	11	36	29	65	14	14	A-6a	6								
		SS-3	4.5	6.0	4.0	5.5	15			26	16	10	36	24	60	13	11	A-4a	5								
		SS-4	6.0	7.5	5.5	7.0	14			2.5							22	14	A-6a								
13	B 013-0 21	SS-1	1.5	3.0	0.3	1.8	80	12	4.25								24	6	A-1-b	0	1433						
		SS-2	3.0	4.5	1.8	3.3	33			24	17	7	21	13	34	9	10	A-2-4	0								
		SS-3	4.5	6.0	3.3	4.8	14			24	14	10	38	29	67	12	10	A-4a	6								
		SS-4	6.0	7.5	4.8	6.3	12			2.5							15	10	A-4a	8							
14	B 014-0 21	SS-1	1.5	3.0	-0.4	1.1	34	20	4.5	27	17	10	31	24	55	5	12	A-4a	4	4767							
		SS-2	3.0	4.5	1.1	2.6	20										13	10	A-4a	8							
		SS-3	4.5	6.0	2.6	4.1	20			24	16	8	25	16	41	11	11	A-4a	1								
		SS-4	6.0	7.5	4.1	5.6	23			4.5							11	10	A-4a	8							
15	B 015-0 21	SS-1	1.5	3.0	-1.3	0.2	109	11	3.25								12	6	A-1-b	0	2300						
		SS-2	3.0	4.5	0.2	1.7	15			28	18	10	31	18	49	12	13	A-4a	3								
		SS-3	4.5	6.0	1.7	3.2	11			25	16	9	32	22	54	11	11	A-4a	4								
		SS-4	6.0	7.5	3.2	4.7	12			3.25							13	10	A-4a	8							
16	B 016-0 21	SS-1	1.5	3.0	-2.1	-0.6	50	18	4.5								14	6	A-1-b	0	3767						
		SS-2	3.0	4.5	-0.6	0.9	18			25	16	9	36	24	60	11	11	A-4a	5								
		SS-3	4.5	6.0	0.9	2.4	25			25	16	9	34	24	58	12	11	A-4a	5								
		SS-4	6.0	7.5	2.4	3.9	25			4.5							13	10	A-4a	8							
17	B 017-0 21	SS-1	1.5	3.0	-2.7	-1.2	23	16	4.5	32	19	13	33	24	57	13	14	A-6a	6	193							
		SS-2	3.0	4.5	-1.2	0.3	29			26	17	9	35	23	58	11	12	A-4a	5								
		SS-3	4.5	6.0	0.3	1.8	20			4.5							11	14	A-6a	10							
		SS-4	6.0	7.5	1.8	3.3	16			3.75							11	14	A-6a	10							
18	B 018-0 21	SS-1	1.5	3.0	0.4	1.9	20	10	4.5	36	20	16	29	26	55	12	16	A-6b	6	213							
		SS-2	3.0	4.5	1.9	3.4	15			50	22	28	33	40	73	25	19	A-7-6	17								
		SS-3	4.5	6.0	3.4	4.9	10			1.75							27	18	A-7-6	16							
		SS-4	6.0	7.5	4.9	6.4	12			1.5							22	18	A-7-6	16							



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
19	B 019-0	SS-1	1.5	3.0	-0.7	0.8	19	15	4.5	31	17	14	36	25	61	13	14	A-6a	7	633							
		SS-2	3.0	4.5	0.8	2.3	19		4.5	28	17	11	38	26	64	14	14	A-6a	6								
		SS-3	4.5	6.0	2.3	3.8	15		4.5									16	14	A-6a	10						
		SS-4	6.0	7.5	3.8	5.3	15		4.5									15	14	A-6a	10						
20	B 020-0	SS-1	1.5	3.0	-0.5	1.0	29	10	4.5	26	17	9	35	25	60	12	12	A-4a	5	393							
		SS-2	3.0	4.5	1.0	2.5	23		4.5	27	16	11	34	25	59	12	14	A-6a	5								
		SS-3	4.5	6.0	2.5	4.0	10		3.75									13	16	A-6b	16						
		SS-4	6.0	7.5	4.0	5.5	11		2.25									14	16	A-6b	16						
21	B 021-0	SS-1	1.5	3.0	-0.4	1.1	20	16	4.5	32	18	14	36	30	66	13	14	A-6a	8	293							
		SS-2	3.0	4.5	1.1	2.6	23		4.5	29	18	11	37	28	65	14	14	A-6a	6								
		SS-3	4.5	6.0	2.6	4.1	20		4.5									15	14	A-6a	10						
		SS-4	6.0	7.5	4.1	5.6	16		4.5									15	14	A-6a	10						
22	B 022-0	SS-1	1.5	3.0	0.2	1.8	16	10										6	6	A-1-b	0						
		SS-2	3.0	4.5	1.8	3.3	12			NP	NP	NP	14	4	18	6	6	A-1-b	0								
		SS-3	4.5	6.0	3.3	4.8	11		4.25	38	19	19	39	36	75	18	16	A-6b	12	240							
		SS-4	6.0	7.5	4.8	6.3	10											11	6	A-1-a	0						
23	B 023-0	SS-1	1.5	3.0	0.3	1.8	20	18	4.5									14	16	A-6b	16	533					
		SS-2	3.0	4.5	1.8	3.3	18		4.5	35	19	16	31	27	58	16	16	A-6b	7								
		SS-3	4.5	6.0	3.3	4.8	30		4.25									11	16	A-6b	16						
		SS-4	6.0	7.5	4.8	6.3	34			32	18	14	22	16	38	9	14	A-6a	2								
24	B 024-0	SS-1	1.5	3.0	0.2	1.7	22	22	4.5									16	16	A-6b	16						
		SS-2	3.0	4.5	1.7	3.2	26		4.5	26	16	10	37	25	62	11	11	A-4a	5	1967							
		SS-3	4.5	6.0	3.2	4.7	29		4.5	25	16	9	38	22	60	10	11	A-4a	5								
		SS-4	6.0	7.5	4.7	6.2	35		4.5									10	10	A-4a	8						
25	B 025-0	SS-1	1.5	3.0	1.2	2.7	14	5	4.5									10	10	A-4a	8						
		SS-2	3.0	4.5	2.7	4.2	5		2.5	25	15	10	39	23	62	13	10	A-4a	5	1227							
		SS-3	4.5	6.0	4.2	5.7	8		3.75	24	15	9	38	24	62	13	10	A-4a	5								
		SS-4	6.0	7.5	5.7	7.2	10		4									13	10	A-4a							
26	B 026-0	SS-1	1.5	3.0	0.1	1.6	8	5	3	32	17	15	37	32	69	17	14	A-6a	9	673						12"	
		SS-2	3.0	4.5	1.6	3.1	10		3.75	33	19	14	38	33	71	19	14	A-6a	9								
		SS-3	4.5	6.0	3.1	4.6	5		3.5									13	14	A-6a	10						
		SS-4	6.0	7.5	4.6	6.1	5		2.5									17	14	A-6a	10						
27	B 027-0	SS-1	1.5	3.0	-1.1	0.4	7	7		NP	NP	NP	19	14	33	10	10	A-2-4	0						15"		
		SS-2	3.0	4.5	0.4	1.9	19			34	20	14	27	21	48	12	15	A-6a	4	807							
		SS-3	4.5	6.0	1.9	3.4	18											16	14	A-6a	10						
		SS-4	6.0	7.5	3.4	4.9	18											14	14	A-6a	10						



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
28	B 028-0 21	SS-1	1.5	3.0	3.1	4.6	12	12	4.5	34	17	17	34	29	63	15	16	A-6b	8	200							
		SS-2	3.0	4.5	4.6	6.1	13		4.5	34	18	16	36	35	71	16	16	A-6b	10								
		SS-3	4.5	6.0	6.1	7.6	13		4.5										15	16	A-6b						
		SS-4	6.0	7.5	7.6	9.1	16		4.5										11	16	A-6b						
29	B 029-0 21	SS-1	1.5	3.0	1.4	2.9	9	9	4.5	32	18	14	39	33	72	17	14	A-6a	9	280							
		SS-2	3.0	4.5	2.9	4.4	16		2.25	35	18	17	36	34	70	20	16	A-6b	10								
		SS-3	4.5	6.0	4.4	5.9	9		2.75										21	16	A-6b	16					
		SS-4	6.0	7.5	5.9	7.4	14		3										22	16	A-6b						
30	B 030-0 21	SS-1	1.5	3.0	3.0	4.5	12	3		NP	NP	NP	14	6	20	7	6	A-1-b	0								
		SS-2A	3.0	3.5	4.5	5.0	5												7	6	A-1-b	0					
		SS-2B	3.5	4.5	5.0	6.0	5			31	17	14	34	29	63	20	14	A-6a	7								
		SS-3	4.5	6.0	6.0	7.5	3		1.25										26	14	A-6a						
31	B 031-0 21	SS-1	1.5	3.0	2.8	4.3	7	7	4.25	32	17	15	39	35	74	18	14	A-6a	10								
		SS-2	3.0	4.5	4.3	5.8	8		4	33	16	17	41	32	73	17	16	A-6b	10								
		SS-3	4.5	6.0	5.8	7.3	9		1.75										17	14	A-6a						
		SS-4	6.0	7.5	7.3	8.8	11		2.25										13	14	A-6a						
32	B 032-0 21	SS-1	1.5	3.0	1.8	3.3	18	11	4.5	24	15	9	38	22	60	11	10	A-4a	5	240							
		SS-2	3.0	4.5	3.3	4.8	12		4.5	24	14	10	39	26	65	11	10	A-4a	6								
		SS-3	4.5	6.0	4.8	6.3	11		4.5										10	10	A-4a	8					
		SS-4	6.0	7.5	6.3	7.8	8		4.5										10	10	A-4a						
33	B 033-0 21	SS-1	2.5	4.0	1.5	3.0	21	13	4.5	27	17	10	36	27	63	11	12	A-4a	6								
		SS-2	5.0	6.5	4.0	5.5	13		3.25	33	19	14	40	32	72	16	14	A-6a	9								
		SS-3	7.5	9.0	6.5	8.0	16		4										21	18	A-7-6						
		ST-1	10.0	12.0	9.0	11.0			3.5										24	18	A-7-6						
34	B 034-0 21	SS-1	2.5	4.0	1.7	3.2	16	16	4.5	37	18	19	38	38	76	17	16	A-6b	12								
		SS-2	5.0	6.5	4.2	5.7	16		4.5										13	16	A-6b	16					
		SS-3	7.5	9.0	6.7	8.2	26		4.5	32	18	14	36	33	69	15	14	A-6a									
		SS-4	10.0	11.5	9.2	10.7	20		4.5										20	14	A-6a						
35	B 035-0 21	SS-1	1.5	3.0	-1.0	0.5	16	16	4.5										13	16	A-6b	16					
		SS-2	3.0	4.5	0.5	2.0	25		4.5	34	17	17	35	35	70	14	16	A-6b	10	80							
		SS-3	4.5	6.0	2.0	3.5	28		4.5										16	16	A-6b	16					
		SS-4	6.0	7.5	3.5	5.0	28		4.5	34	22	12	47	37	84	19	17	A-6a	9								
36	B 036-0 21	SS-1	1.5	3.0	-0.2	1.3	14	8	4.5	24	14	10	34	23	57	10	10	A-4a	4	600							
		SS-2	3.0	4.5	1.3	2.8	13		4.5	23	14	9	38	24	62	12	10	A-4a	5								
		SS-3	4.5	6.0	2.8	4.3	16		4.5										11	10	A-4a	8					
		SS-4	6.0	7.5	4.3	5.8	8		4										11	10	A-4a	8					



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
37	B 037-0 21	SS-1	1.5	3.0	0.8	2.4	18	18	4.5	23	14	9	39	25	64	10	10	A-4a	6	780							
		SS-2	3.0	4.5	2.4	3.9	20		4.5	23	13	10	36	24	60	9	10	A-4a	5								
		SS-3	4.5	6.0	3.9	5.4	25		4.5									10	10	A-4a	8						
		SS-4	6.0	7.5	5.4	6.9	38		4.5									9	10	A-4a							
38	B 038-0 21	SS-1	2.5	4.0	1.4	2.9	20	17	4.5	26	16	10	38	24	62	12	11	A-4a	5								
		SS-2	5.0	6.5	3.9	5.4	17		4.5	37	21	16	41	35	76	18	16	A-6b	10								
		SS-3	7.5	9.0	6.4	7.9	16		4.5									20	16	A-6b							
		SS-4	10.0	11.5	8.9	10.4	16		4									26	16	A-6b							
39	B 039-0 21	SS-1	1.5	3.0	0.5	2.0	18	14	4.5	27	16	11	34	28	62	12	14	A-6a	6	400							
		SS-2	3.0	4.5	2.0	3.5	16		4.5									12	14	A-6a	10						
		SS-3	4.5	6.0	3.5	5.0	14		4.5									13	14	A-6a	10						
		SS-4	6.0	7.5	5.0	6.5	16		4.5	27	14	13	35	27	62	11	14	A-6a									
40	B 040-0 21	SS-1	1.5	3.0	0.4	1.9	26	26	4.5	23	14	9	35	25	60	8	10	A-4a	5	6033							
		SS-2	3.0	4.5	1.9	3.4	33		4.5	27	16	11	37	28	65	11	14	A-6a	6								
		SS-3	4.5	6.0	3.4	4.9	29		4.5									10	14	A-6a	10						
		SS-4	6.0	7.5	4.9	6.4	33		4.5									13	14	A-6a	10						
41	B 041-0 21	SS-1	1.5	3.0	1.1	2.6	11	11	4.5	25	15	10	38	27	65	12	10	A-4a	6	2633			N ₆₀				
		SS-2	3.0	4.5	2.6	4.1	19		4.5	32	17	15	36	33	69	13	14	A-6a	9								
		SS-3	4.5	6.0	4.1	5.6	19		4.5									14	14	A-6a	10						
		SS-4	6.0	7.5	5.6	7.1	27		4.5									13	14	A-6a							
42	B 042-0 21	SS-1	1.5	3.0	0.8	2.3	20	8										7	10	A-2-4	0						
		SS-2	3.0	4.5	2.3	3.8	16		4.5	30	17	13	37	28	65	14	14	A-6a	7	1280							
		SS-3	4.5	6.0	3.8	5.3	8		2.75									22	14	A-6a	10						
		SS-4	6.0	7.5	5.3	6.8	18		3.5	53	25	28	40	49	89	28	22	A-7-6									
43	B 043-0 21	SS-1	1.5	3.0	1.7	3.2	27	17	4.5	28	17	11	39	27	66	14	14	A-6a	7	160							
		SS-2	3.0	4.5	3.2	4.7	22		4.5									13	14	A-6a	10						
		SS-3	4.5	6.0	4.7	6.2	17		4.5	25	18	7	32	21	53	12	13	A-4a	4								
		SS-4	6.0	7.5	6.2	7.7	20		4.5									10	10	A-4a							

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 43

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	12"

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	7
-------------------	----------

% Samples within 6 feet of subgrade			
N₆₀ ≤ 5	4%	HP ≤ 0.5	0%
N₆₀ < 12	18%	0.5 < HP ≤ 1	0%
12 ≤ N₆₀ < 15	11%	1 < HP ≤ 2	4%
N₆₀ ≥ 20	44%	HP > 2	81%
M+	4%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

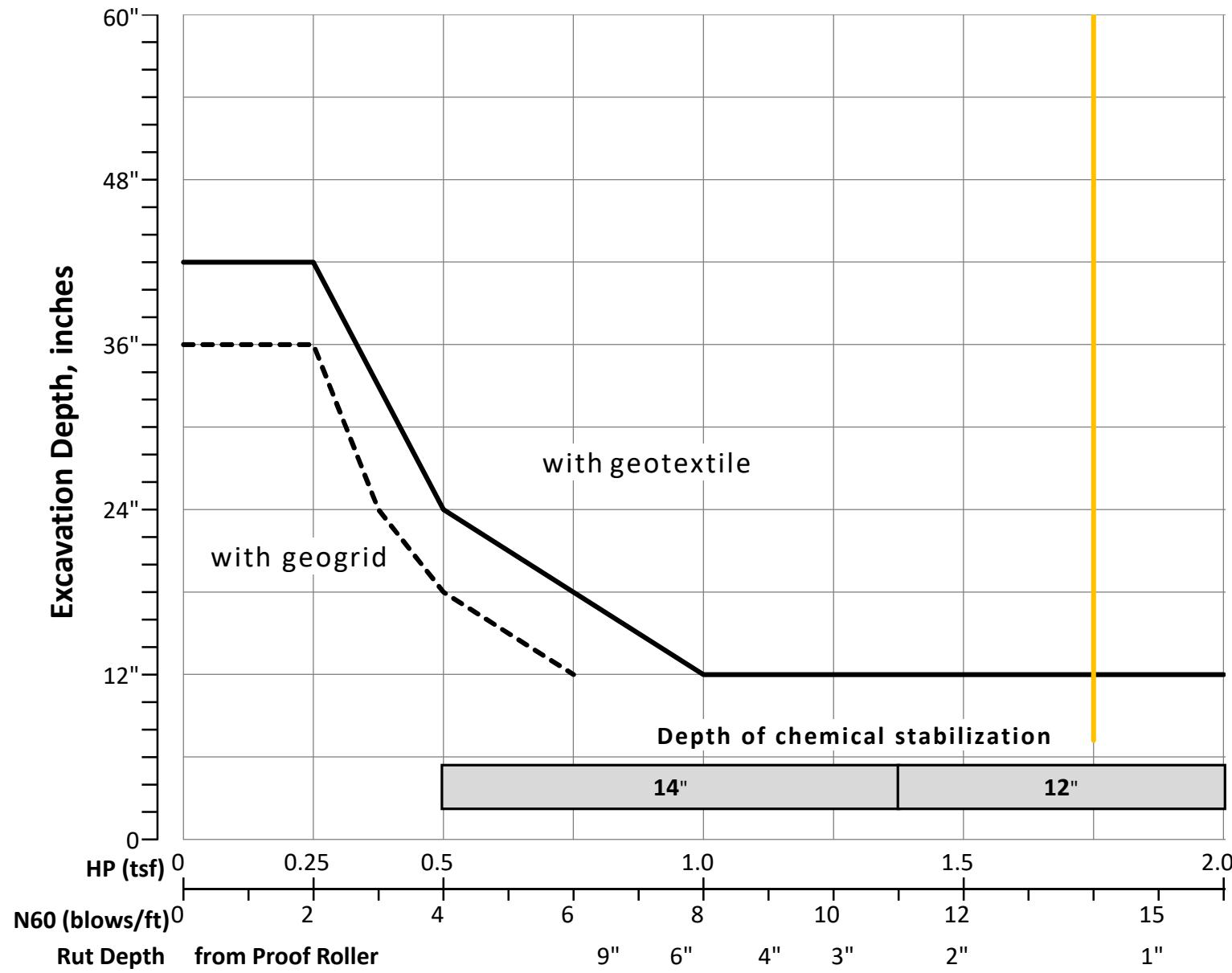
% Proposed Subgrade Surface	
Unstable & Unsuitable	11%
Unstable	11%
Unsuitable	0%

	N₆₀	N_{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M_c	M_{opt}	GI
Average	21	14	4.03	29	17	12	34	26	60	14	13	7
Maximum	109	26	4.50	53	25	28	47	49	89	28	22	17
Minimum	3	3	1.25	22	13	7	6	2	8	4	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	4	12	3	0	0	0	0	0	62	0	0	54	27	0	8	0	0	170
Percent	0%	2%	7%	2%	0%	0%	0%	0%	0%	36%	0%	0%	32%	16%	0%	5%	0%	0%	100%
% Rock Granular Cohesive	0%																		100%
Surface Class Count	0	3	10	3	0	0	0	0	0	37	0	0	26	11	0	1	0	0	91
Surface Class Percent	0%	3%	11%	3%	0%	0%	0%	0%	0%	41%	0%	0%	29%	12%	0%	1%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.03	0.50	<input type="checkbox"/> HP
14.02	6.00	<input type="checkbox"/> N60L

Average HP

Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION
OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES
Geotechnical Bulletin GB1

FRA-071/270-28.27/25.99A

105435

IR-71 NB

NEAS, INC.

Prepared By: MWJ
Date prepared: Friday, September 09, 2022

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614-714-0299
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NO. OF BORINGS: **20**



#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-027-0-21	I-71	127+88	104	RT	CME 45B	82	913.7	910.1	3.7 C
2	B-001-0-21	I-71	130+48	62	RT	CME 45B	82	913.0	910.8	2.2 C
3	B-002-0-21	I-71	134+73	77	RT	CME 45B	82	914.1	912.2	1.9 C
4	B-033-0-21	I-71	137+81	9	RT	CME 55T / CME 75T	63 / 79	913.3	912.2	1.0 C
5	B-003-0-21	I-71	138+60	75	RT	CME 45B	82	913.7	912.0	1.7 C
6	B-038-0-21	I-71	139+20	11	RT	CME 75T	82	912.8	911.7	1.1 C
7	B-004-0-21	I-71	142+56	79	RT	CME 45B	82	911.1	909.7	1.4 C
8	B-005-0-21	I-71	146+32	58	RT	CME 45B	82	908.2	906.0	2.2 C
9	B-006-0-21	I-71	150+94	91	RT	CME 45B	82	903.0	901.4	1.6 C
10	B-007-0-21	I-71	154+66	89	RT	CME 45B	82	899.4	897.7	1.7 C
11	B-008-0-21	I-71	158+69	85	RT	CME 45B	82	895.2	893.6	1.6 C
12	B-009-0-21	I-71	162+72	91	RT	CME 45B	82	891.0	889.6	1.4 C
13	B-010-0-21	I-71	166+56	86	RT	CME 45B	82	887.8	885.9	1.9 C
14	B-011-0-21	I-71	170+65	86	RT	CME 45B	82	885.9	884.7	1.2 C
15	B-012-0-21	I-71	174+56	86	RT	CME 45B	82	886.8	886.3	0.5 C
16	B-013-0-21	I-71	178+53	86	RT	CME 45B	82	889.2	888.0	1.2 C
17	B-014-0-21	I-71	182+61	85	RT	CME 45B	82	891.6	889.7	1.9 C
18	B-015-0-21	I-71	186+58	84	RT	CME 45B	82	894.1	891.3	2.8 C
19	B-016-0-21	I-71	190+59	84	RT	CME 45B	82	896.6	893.0	3.6 C
20	B-017-0-21	I-71	194+58	96	RT	CME 45B	82	898.8	894.7	4.2 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable	
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable	
1	B 027-0 21	SS-1	1.5	3.0	-2.2	-0.7	7	18	NP 34 16 14	NP	NP	NP	19	14	33	10	10	A-2-4	0	807					
		SS-2	3.0	4.5	-0.7	0.8	19			34	20	14	27	21	48	12	15	A-6a	4						
		SS-3	4.5	6.0	0.8	2.3	18									16	14	A-6a	10						
		SS-4	6.0	7.5	2.3	3.8	18									14	14	A-6a	10						
2	B 001-0 21	SS-1	1.5	3.0	-0.7	0.8	19	19	NP 4.5 4.5 3.75	NP	NP	NP	24	15	39	10	11	A-4a	1	233					
		SS-2	3.0	4.5	0.8	2.3	20			4.5	39	19	20	43	41	84	21	16	A-6b	12	Mc				
		SS-3	4.5	6.0	2.3	3.8	29			4.5							18	16	A-6b	16					
		SS-4	6.0	7.5	3.8	5.3	26			3.75							25	16	A-6b	16					
3	B 002-0 21	SS-1	1.5	3.0	-0.4	1.1	27	11	4.5 4.5 4.5 2.75	27	17	10	39	27	66	13	12	A-4a	6	400					
		SS-2	3.0	4.5	1.1	2.6	19			4.5	28	17	11	35	22	57	11	14	A-6a	5					
		SS-3	4.5	6.0	2.6	4.1	15			4.5							19	14	A-6a	10					
		SS-4	6.0	7.5	4.1	5.6	11			2.75							21	18	A-7-6	16					
4	B 033-0 21	SS-1	2.5	4.0	1.5	3.0	21	13	4.5 3.25 4 3.5	27	17	10	36	27	63	11	12	A-4a	6	218					
		SS-2	5.0	6.5	4.0	5.5	13			3.25	33	19	14	40	32	72	16	14	A-6a	9					
		SS-3	7.5	9.0	6.5	8.0	16			4							21	18	A-7-6						
		ST-1	10.0	12.0	9.0	11.0				3.5							24	18	A-7-6						
5	B 003-0 21	SS-1	1.5	3.0	-0.2	1.3	35	18	NP NP 2.75 2.25							9	6	A-1-a	0	100					
		SS-2	3.0	4.5	1.3	2.8	30			NP	NP	NP	6	2	8	4	6	A-1-a	0						
		SS-3	4.5	6.0	2.8	4.3	18			2.75	32	18	14	34	29	63	18	14	A-6a	7					
		SS-4	6.0	7.5	4.3	5.8	22			2.25							21	14	A-6a	10					
6	B 038-0 21	SS-1	2.5	4.0	1.4	2.9	20	17	4.5 4.5 4.5 4	26	16	10	38	24	62	12	11	A-4a	5	100					
		SS-2	5.0	6.5	3.9	5.4	17			4.5	37	21	16	41	35	76	18	16	A-6b	10					
		SS-3	7.5	9.0	6.4	7.9	16			4.5							20	16	A-6b						
		SS-4	10.0	11.5	8.9	10.4	16			4							26	16	A-6b						
7	B 004-0 21	SS-1	1.5	3.0	0.1	1.6	60	25	4.5 4.5 3.5 3.5							9	6	A-1-b	0	20					
		SS-2	3.0	4.5	1.6	3.1	35			4.5	34	19	15	40	33	73	17	14	A-6a	10	Mc				
		SS-3	4.5	6.0	3.1	4.6	29			3.5	32	17	15	36	32	68	16	14	A-6a	9					
		SS-4	6.0	7.5	4.6	6.1	25			3.5							24	18	A-7-6	16					
8	B 005-0 21	SS-1	1.5	3.0	-0.7	0.8	35	20	4.5 4.5 4 4.25							8	6	A-1-b	0	260					
		SS-2	3.0	4.5	0.8	2.3	23			4.5	26	17	9	34	22	56	14	12	A-4a	4					
		SS-3	4.5	6.0	2.3	3.8	20			4							15	10	A-4a	8					
		SS-4	6.0	7.5	3.8	5.3	23			4.25							12	10	A-4a	8					
9	B 006-0 21	SS-1	1.5	3.0	-0.1	1.4	18	18	4.5 4.5 4.5 4.5	24	16	8	35	24	59	13	11	A-4a	5	580					
		SS-2	3.0	4.5	1.4	2.9	30			4.5	25	16	9	27	17	44	9	11	A-4a	2					
		SS-3	4.5	6.0	2.9	4.4	37										9	6	A-1-b	0					
		SS-4	6.0	7.5	4.4	5.9	41			4.5							10	10	A-4a	8					



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)	
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable		
10	B 007-0 21	SS-1	1.5	3.0	-0.2	1.3	27	10	4.5 4.25 2.25							5	6	A-1-b	0	1147						
		SS-2	3.0	4.5	1.3	2.8	20			23	16	7	27	15	42	9	11	A-4a	1							
		SS-3	4.5	6.0	2.8	4.3	11			23	15	8	31	19	50	11	10	A-4a	3							
		SS-4	6.0	7.5	4.3	5.8	10									12	10	A-4a	8							
11	B 008-0 21	SS-1	1.5	3.0	-0.1	1.4	41	19	4.5 4.5 4 4.25	23	16	7	34	22	56	9	11	A-4a	4	4267						
		SS-2	3.0	4.5	1.4	2.9	19			23	15	8	36	22	58	10	10	A-4a	5							
		SS-3	4.5	6.0	2.9	4.4	19									12	10	A-4a	8							
		SS-4	6.0	7.5	4.4	5.9	22									11	10	A-4a	8							
12	B 009-0 21	SS-1	1.5	3.0	0.1	1.6	33	12	4.5 4.5 4.5 1.75							9	10	A-4a	8	>8000						
		SS-2	3.0	4.5	1.6	3.1	25			24	14	10	37	23	60	9	10	A-4a	5							
		SS-3	4.5	6.0	3.1	4.6	12			22	15	7	37	21	58	11	10	A-4a	5							
		SS-4	6.0	7.5	4.6	6.1	15									13	10	A-4a	8							
13	B 010-0 21	SS-1	1.5	3.0	-0.4	1.1	52	20	4.5 4.5 4.5 3.5							5	6	A-1-a	0							
		SS-2	3.0	4.5	1.1	2.6	20			33	25	8	35	14	49	21	20	A-4a	3	5767						
		SS-3	4.5	6.0	2.6	4.1	27									12	10	A-4a	8							
		SS-4	6.0	7.5	4.1	5.6	26			33	19	14	43	31	74	20	14	A-6a	9							
14	B 011-0 21	SS-1	1.5	3.0	0.3	1.8	78	16	4.5 4.5 4.5 1.5							5	6	A-1-b	0							
		SS-2	3.0	4.5	1.8	3.3	18			26	16	10	37	30	67	12	11	A-4a	6	440						
		SS-3	4.5	6.0	3.3	4.8	16			28	17	11	35	27	62	13	14	A-6a	6							
		SS-4	6.0	7.5	4.8	6.3	18									28	14	A-6a	10							
15	B 012-0 21	SS-1	1.5	3.0	1.0	2.5	52	14	4.5 4.5 4.5 2.5							10	6	A-1-b	0							
		SS-2	3.0	4.5	2.5	4.0	15			28	17	11	36	29	65	14	14	A-6a	6	220						
		SS-3	4.5	6.0	4.0	5.5	15			26	16	10	36	24	60	13	11	A-4a	5							
		SS-4	6.0	7.5	5.5	7.0	14									22	14	A-6a								
16	B 013-0 21	SS-1	1.5	3.0	0.3	1.8	80	12	4.25 4.25 4.25 2.5							24	6	A-1-b	0							
		SS-2	3.0	4.5	1.8	3.3	33			24	17	7	21	13	34	9	10	A-2-4	0	1433						
		SS-3	4.5	6.0	3.3	4.8	14			24	14	10	38	29	67	12	10	A-4a	6							
		SS-4	6.0	7.5	4.8	6.3	12									15	10	A-4a	8							
17	B 014-0 21	SS-1	1.5	3.0	-0.4	1.1	34	20	4.5 4.5 4.5 4.5	27	17	10	31	24	55	5	12	A-4a	4	4767						
		SS-2	3.0	4.5	1.1	2.6	20									13	10	A-4a	8							
		SS-3	4.5	6.0	2.6	4.1	20			24	16	8	25	16	41	11	11	A-4a	1							
		SS-4	6.0	7.5	4.1	5.6	23									11	10	A-4a	8							
18	B 015-0 21	SS-1	1.5	3.0	-1.3	0.2	109	11	4.5 4.5 3.25 3.25							12	6	A-1-b	0							
		SS-2	3.0	4.5	0.2	1.7	15			28	18	10	31	18	49	12	13	A-4a	3	2300						
		SS-3	4.5	6.0	1.7	3.2	11			25	16	9	32	22	54	11	11	A-4a	4							
		SS-4	6.0	7.5	3.2	4.7	12									13	10	A-4a	8							



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
19	B 016-0 21	SS-1	1.5	3.0	-2.1	-0.6	50	18	4.5							14	6	A-1-b	0								
		SS-2	3.0	4.5	-0.6	0.9	18			25	16	9	36	24	60	11	11	A-4a	5	3767							
		SS-3	4.5	6.0	0.9	2.4	25			25	16	9	34	24	58	12	11	A-4a	5								
		SS-4	6.0	7.5	2.4	3.9	25			18	4.5						13	10	A-4a	8							
20	B 017-0 21	SS-1	1.5	3.0	-2.7	-1.2	23	16	3.75	32	19	13	33	24	57	13	14	A-6a	6	193							
		SS-2	3.0	4.5	-1.2	0.3	29			26	17	9	35	23	58	11	12	A-4a	5								
		SS-3	4.5	6.0	0.3	1.8	20			4.5							11	14	A-6a	10							
		SS-4	6.0	7.5	1.8	3.3	16			3.75							11	14	A-6a	10							

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 20

Geotechnical Consultant: NEAS, INC.

Prepared By: MWJ

Date prepared: 9/9/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	NA

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	7
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% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ < 12	6%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	8%	1 < HP ≤ 2	3%
N ₆₀ ≥ 20	59%	HP > 2	75%
M+	4%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

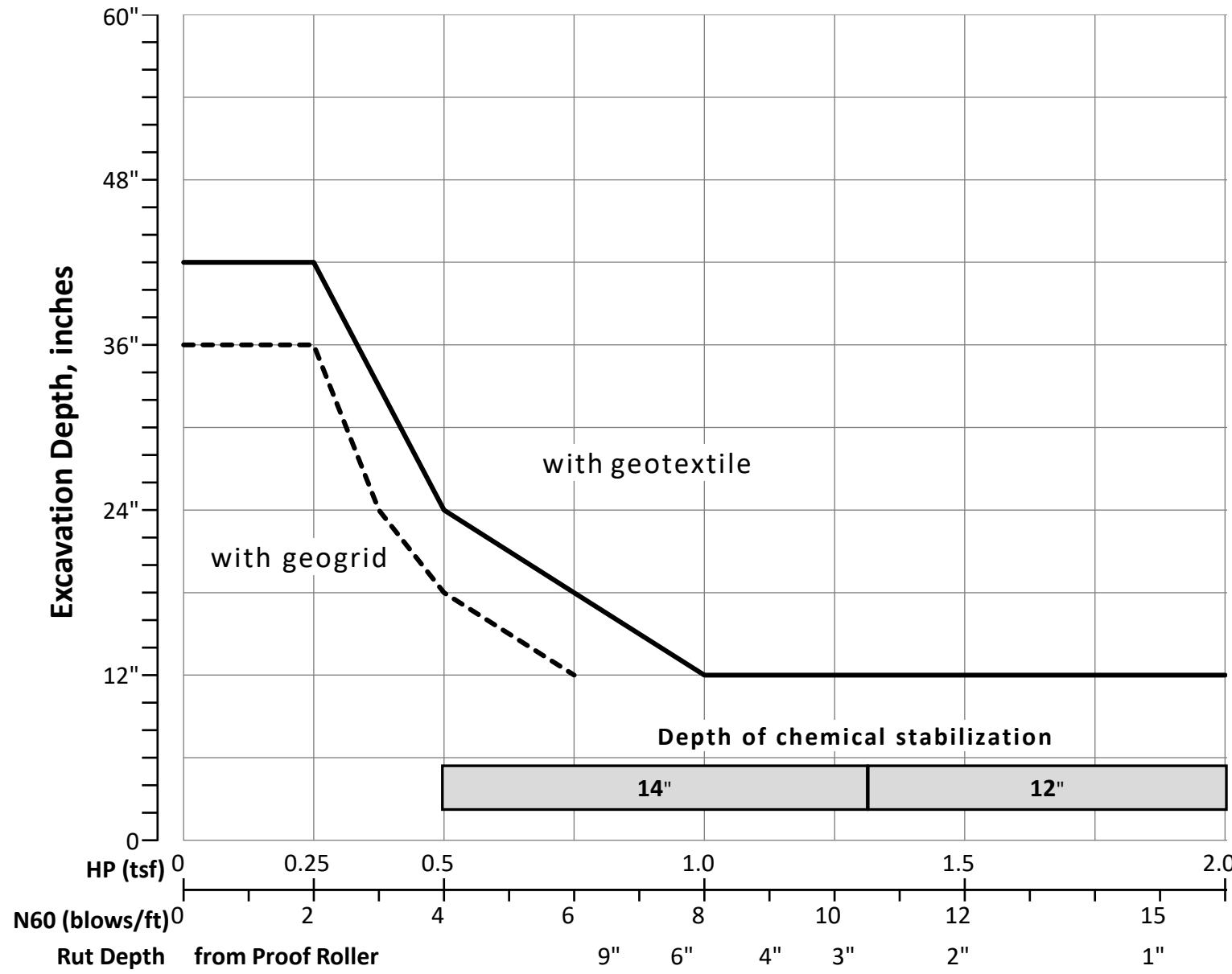
% Proposed Subgrade Surface	
Unstable & Unsuitable	8%
Unstable	8%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	26	16	4.01	28	17	10	34	24	57	14	12	6
Maximum	109	25	4.50	39	25	20	43	41	84	28	20	16
Minimum	7	10	1.50	22	14	7	6	2	8	4	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	3	8	1	0	0	0	0	0	38	0	0	17	6	0	4	0	0	77
Percent	0%	4%	10%	1%	0%	0%	0%	0%	0%	49%	0%	0%	22%	8%	0%	5%	0%	0%	100%
% Rock Granular Cohesive	0%	65%										35%						100%	
Surface Class Count	0	3	8	2	0	0	0	0	0	26	0	0	10	2	0	0	0	0	51
Surface Class Percent	0%	6%	16%	4%	0%	0%	0%	0%	0%	51%	0%	0%	20%	4%	0%	0%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization



Average HP
Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****North I-71 with high sulfate contents****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 07, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**9**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-008-0-21	I-71	158+69	85	RT	CME 45B	82	895.2	893.6	1.6 C
2	B-009-0-21	I-71	162+72	91	RT	CME 45B	82	891.0	889.6	1.4 C
3	B-010-0-21	I-71	166+56	86	RT	CME 45B	82	887.8	885.9	1.9 C
4	B-011-0-21	I-71	170+65	86	RT	CME 45B	82	885.9	884.7	1.2 C
5	B-012-0-21	I-71	174+56	86	RT	CME 45B	82	886.8	886.3	0.5 C
6	B-013-0-21	I-71	178+53	86	RT	CME 45B	82	889.2	888.0	1.2 C
7	B-014-0-21	I-71	182+61	85	RT	CME 45B	82	891.6	889.7	1.9 C
8	B-015-0-21	I-71	186+58	84	RT	CME 45B	82	894.1	891.3	2.8 C
9	B-016-0-21	I-71	190+59	84	RT	CME 45B	82	896.6	893.0	3.6 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
1	B 008-0 21	SS-1	1.5	3.0	-0.1	1.4	41	19	4.5	23	16	7	34	22	56	9	11	A-4a	4	4267							
		SS-2	3.0	4.5	1.4	2.9	19		4.5	23	15	8	36	22	58	10	10	A-4a	5								
		SS-3	4.5	6.0	2.9	4.4	19		4									A-4a	8								
		SS-4	6.0	7.5	4.4	5.9	22		4.25									A-4a	8								
2	B 009-0 21	SS-1	1.5	3.0	0.1	1.6	33	12	4.5									9	10	A-4a	8	>8000					
		SS-2	3.0	4.5	1.6	3.1	25		4.5	24	14	10	37	23	60	9	10	A-4a	5								
		SS-3	4.5	6.0	3.1	4.6	12		4.5	22	15	7	37	21	58	11	10	A-4a	5								
		SS-4	6.0	7.5	4.6	6.1	15		1.75									13	10	A-4a	8						
3	B 010-0 21	SS-1	1.5	3.0	-0.4	1.1	52	20										5	6	A-1-a	0						
		SS-2	3.0	4.5	1.1	2.6	20		4.5	33	25	8	35	14	49	21	20	A-4a	3	5767							
		SS-3	4.5	6.0	2.6	4.1	27		4.5									12	10	A-4a	8						
		SS-4	6.0	7.5	4.1	5.6	26		3.5	33	19	14	43	31	74	20	14	A-6a	9								
4	B 011-0 21	SS-1	1.5	3.0	0.3	1.8	78	16										5	6	A-1-b	0						
		SS-2	3.0	4.5	1.8	3.3	18		4.5	26	16	10	37	30	67	12	11	A-4a	6	440							
		SS-3	4.5	6.0	3.3	4.8	16		4.5	28	17	11	35	27	62	13	14	A-6a	6								
		SS-4	6.0	7.5	4.8	6.3	18		1.5									28	14	A-6a	10						
5	B 012-0 21	SS-1	1.5	3.0	1.0	2.5	52	14										10	6	A-1-b	0						
		SS-2	3.0	4.5	2.5	4.0	15		4.5	28	17	11	36	29	65	14	14	A-6a	6	220							
		SS-3	4.5	6.0	4.0	5.5	15		4.5	26	16	10	36	24	60	13	11	A-4a	5								
		SS-4	6.0	7.5	5.5	7.0	14		2.5									22	14	A-6a							
6	B 013-0 21	SS-1	1.5	3.0	0.3	1.8	80	12										24	6	A-1-b	0						
		SS-2	3.0	4.5	1.8	3.3	33			24	17	7	21	13	34	9	10	A-2-4	0	1433							
		SS-3	4.5	6.0	3.3	4.8	14		4.25	24	14	10	38	29	67	12	10	A-4a	6								
		SS-4	6.0	7.5	4.8	6.3	12		2.5									15	10	A-4a	8						
7	B 014-0 21	SS-1	1.5	3.0	-0.4	1.1	34	20	4.5	27	17	10	31	24	55	5	12	A-4a	4	4767							
		SS-2	3.0	4.5	1.1	2.6	20		4.5									13	10	A-4a	8		Mc				
		SS-3	4.5	6.0	2.6	4.1	20		4.5	24	16	8	25	16	41	11	11	A-4a	1								
		SS-4	6.0	7.5	4.1	5.6	23		4.5									11	10	A-4a	8						
8	B 015-0 21	SS-1	1.5	3.0	-1.3	0.2	109	11										12	6	A-1-b	0						
		SS-2	3.0	4.5	0.2	1.7	15		4.5	28	18	10	31	18	49	12	13	A-4a	3	2300							
		SS-3	4.5	6.0	1.7	3.2	11		3.25	25	16	9	32	22	54	11	11	A-4a	4				N ₆₀				
		SS-4	6.0	7.5	3.2	4.7	12		3.25									13	10	A-4a	8						
9	B 016-0 21	SS-1	1.5	3.0	-2.1	-0.6	50	18										14	6	A-1-b	0						
		SS-2	3.0	4.5	-0.6	0.9	18		4.5	25	16	9	36	24	60	11	11	A-4a	5	3767							
		SS-3	4.5	6.0	0.9	2.4	25		4.5	25	16	9	34	24	58	12	11	A-4a	5								
		SS-4	6.0	7.5	2.4	3.9	25		4.5									13	10	A-4a	8						

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 9

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	12"

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	8
-------------------	----------

% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ <12	3%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	14%	1 < HP ≤ 2	6%
N ₆₀ ≥ 20	54%	HP > 2	77%
M+	3%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

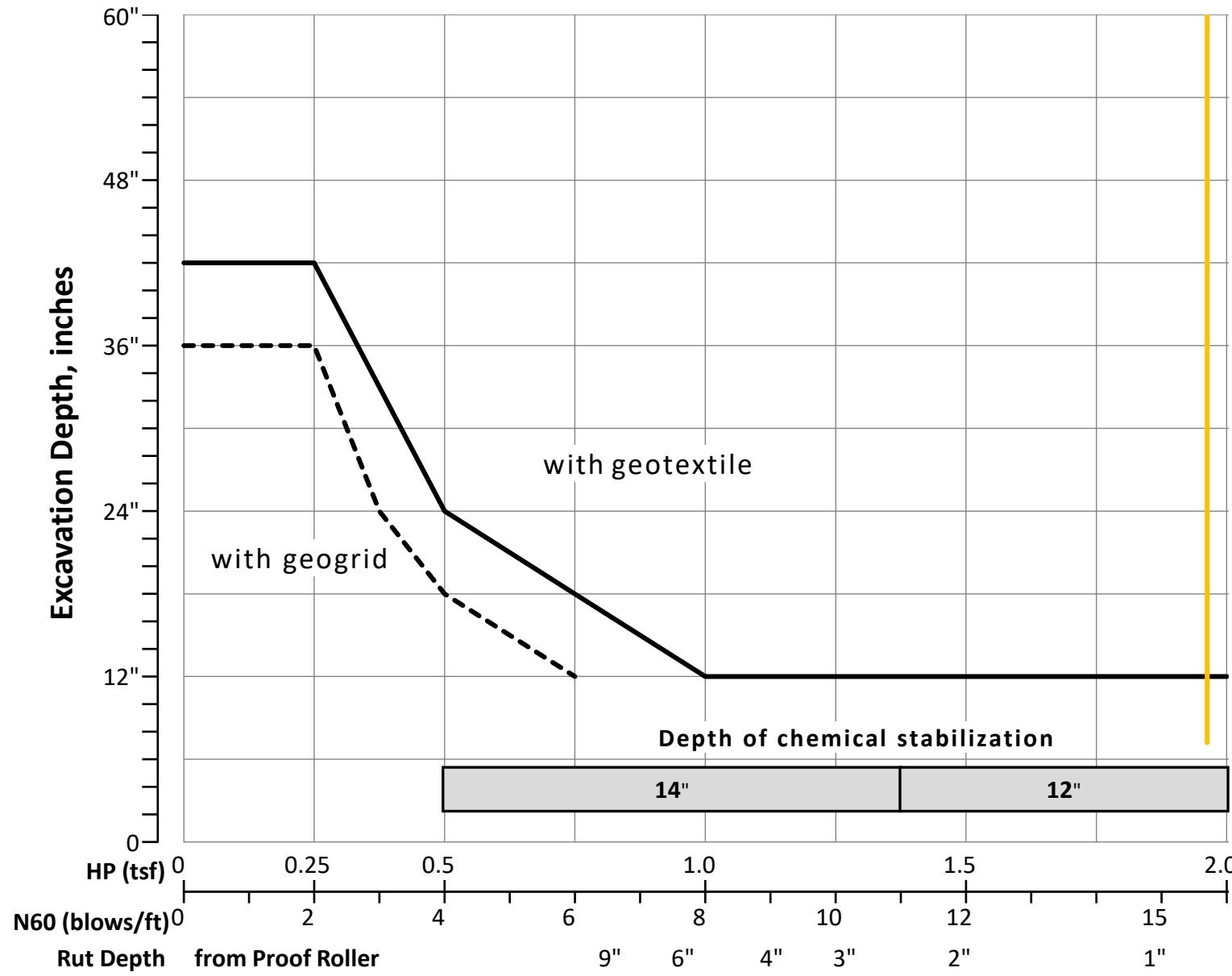
% Proposed Subgrade Surface	
Unstable & Unsuitable	9%
Unstable	9%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	28	16	4.01	26	17	9	34	23	57	13	11	5
Maximum	109	20	4.50	33	25	14	43	31	74	28	20	10
Minimum	11	11	1.50	22	14	7	21	13	34	5	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	1	4	1	0	0	0	0	0	24	0	0	5	0	0	0	0	0	35
Percent	0%	3%	11%	3%	0%	0%	0%	0%	0%	69%	0%	0%	14%	0%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%	86%										14%						100%	
Surface Class Count	0	1	5	1	0	0	0	0	0	15	0	0	1	0	0	0	0	0	23
Surface Class Percent	0%	4%	22%	4%	0%	0%	0%	0%	0%	65%	0%	0%	4%	0%	0%	0%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.01	0.50	<input type="checkbox"/> HP
15.78	6.00	<input type="checkbox"/> N60L

Average HP

Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****Ramp K****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 07, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**7**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-028-0-21	Ramp K	13+63	23	RT	CME 75T	79	921.3	922.8	1.6 F
2	B-029-0-21	Ramp K	16+67	25	RT	CME 75T	79	929.0	928.9	0.1 C
3	B-030-0-21	Ramp K	19+68	27	RT	CME 75T	79	937.1	938.6	1.5 F
4	B-031-0-21	Ramp K	21+80	11	LT	CME 75T	79	940.6	941.9	1.3 F
5	B-032-0-21	Ramp K	24+63	1	RT	CME 75T	79	938.9	939.2	0.3 F
6	B-034-0-21	Ramp K	30+16	2	LT	CME 75T	79	926.7	925.8	0.8 C
7	B-035-0-21	Ramp K	34+09	22	RT	CME 75T	79	915.6	913.2	2.5 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable			
			From	To	From	To																					
1	B 028-0 21	SS-1	1.5	3.0	3.1	4.6	12	12	4.5	34	17	17	34	29	63	15	16	A-6b	8	200							
		SS-2	3.0	4.5	4.6	6.1	13		4.5	34	18	16	36	35	71	16	16	A-6b	10								
		SS-3	4.5	6.0	6.1	7.6	13		4.5									15	16	A-6b							
		SS-4	6.0	7.5	7.6	9.1	16		4.5									11	16	A-6b							
2	B 029-0 21	SS-1	1.5	3.0	1.4	2.9	9	9	4.5	32	18	14	39	33	72	17	14	A-6a	9	280							
		SS-2	3.0	4.5	2.9	4.4	16		2.25	35	18	17	36	34	70	20	16	A-6b	10								
		SS-3	4.5	6.0	4.4	5.9	9		2.75									21	16	A-6b	16						
		SS-4	6.0	7.5	5.9	7.4	14		3									22	16	A-6b							
3	B 030-0 21	SS-1	1.5	3.0	3.0	4.5	12	3		NP	NP	NP	14	6	20	7	6	A-1-b	0								
		SS-2A	3.0	3.5	4.5	5.0	5											7	6	A-1-b	0						
		SS-2B	3.5	4.5	5.0	6.0	5			31	17	14	34	29	63	20	14	A-6a	7								
		SS-3	4.5	6.0	6.0	7.5	3		1.25									26	14	A-6a							
4	B 031-0 21	SS-1	1.5	3.0	2.8	4.3	7	7	4.25	32	17	15	39	35	74	18	14	A-6a	10								
		SS-2	3.0	4.5	4.3	5.8	8		4	33	16	17	41	32	73	17	16	A-6b	10								
		SS-3	4.5	6.0	5.8	7.3	9		1.75									17	14	A-6a							
		SS-4	6.0	7.5	7.3	8.8	11		2.25									13	14	A-6a							
5	B 032-0 21	SS-1	1.5	3.0	1.8	3.3	18	11	4.5	24	15	9	38	22	60	11	10	A-4a	5	240							
		SS-2	3.0	4.5	3.3	4.8	12		4.5	24	14	10	39	26	65	11	10	A-4a	6								
		SS-3	4.5	6.0	4.8	6.3	11		4.5									10	10	A-4a	8						
		SS-4	6.0	7.5	6.3	7.8	8		4.5									10	10	A-4a							
6	B 034-0 21	SS-1	2.5	4.0	1.7	3.2	16	16	4.5	37	18	19	38	38	76	17	16	A-6b	12								
		SS-2	5.0	6.5	4.2	5.7	16		4.5									13	16	A-6b	16						
		SS-3	7.5	9.0	6.7	8.2	26		4.5	32	18	14	36	33	69	15	14	A-6a									
		SS-4	10.0	11.5	9.2	10.7	20		4.5									20	14	A-6a							
7	B 035-0 21	SS-1	1.5	3.0	-1.0	0.5	16	16	4.5									13	16	A-6b	16						
		SS-2	3.0	4.5	0.5	2.0	25		4.5	34	17	17	35	35	70	14	16	A-6b	10	80							
		SS-3	4.5	6.0	2.0	3.5	28		4.5									16	16	A-6b	16						
		SS-4	6.0	7.5	3.5	5.0	28		4.5	34	22	12	47	37	84	19	17	A-6a	9								

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 7

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	No
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	14"

Excavate and Replace Stabilization Options		
Global Geotextile Average(N60L):	12"	0"
Global Geogrid Average(N60L):	0"	0"

Design CBR	6
-------------------	----------

% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	13%	HP ≤ 0.5	0%
N ₆₀ < 12	39%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	26%	1 < HP ≤ 2	9%
N ₆₀ ≥ 20	13%	HP > 2	78%
M+	4%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface		
Average		0"
Maximum		0"
Minimum		0"

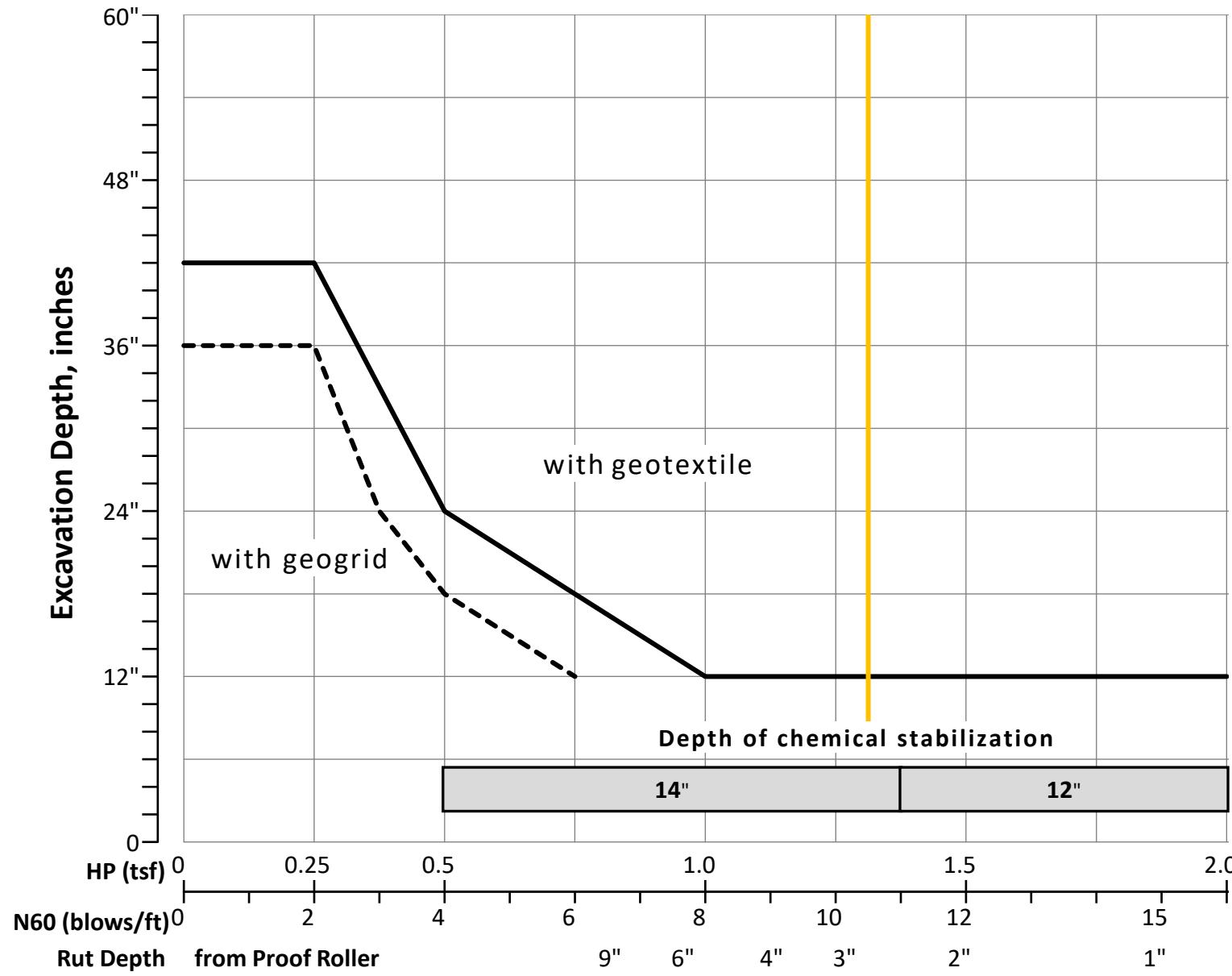
% Proposed Subgrade Surface		
Unstable & Unsuitable	14%	
Unstable	14%	
Unsuitable	0%	

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	14	11	3.92	32	17	15	36	30	66	15	14	9
Maximum	28	16	4.50	37	22	19	47	38	84	26	17	16
Minimum	3	3	1.25	24	14	9	14	6	20	7	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	2	0	0	0	0	0	0	4	0	0	9	13	0	0	0	28	
Percent	0%	0%	7%	0%	0%	0%	0%	0%	0%	14%	0%	0%	32%	46%	0%	0%	0%	100%	
% Rock Granular Cohesive	0%	21%										79%						100%	
Surface Class Count	0	0	0	0	0	0	0	0	0	1	0	0	2	4	0	0	0	7	
Surface Class Percent	0%	0%	0%	0%	0%	0%	0%	0%	0%	14%	0%	0%	29%	57%	0%	0%	0%	100%	



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
3.92	0.50	<input type="checkbox"/> HP
10.57	6.00	<input type="checkbox"/> N60L

Average HP
Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****Ramp M****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 07, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**7**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-018-0-21	Ramp M	84+94	3	LT	CME 45B	82	921.6	920.5	1.1 C
2	B-019-0-21	Ramp M	88+73	46	LT	CME 45B	82	920.4	918.1	2.2 C
3	B-020-0-21	Ramp M	92+48	4	LT	CME 45B	82	917.4	915.4	2.0 C
4	B-021-0-21	Ramp M	96+55	5	RT	CME 45B	82	918.6	916.6	1.9 C
5	B-022-0-21	Ramp M	100+01	3	RT	CME 45B	82	920.6	919.4	1.3 C
6	B-023-0-21	Ramp M	103+72	62	LT	CME 45B	82	923.6	922.4	1.2 C
7	B-024-0-21	Ramp M	107+55	49	LT	CME 45B	82	925.6	924.3	1.3 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable	
1	B 018-0 21	SS-1	1.5	3.0	0.4	1.9	20	10	4.5	36	20	16	29	26	55	12	16	A-6b	6	213					
		SS-2	3.0	4.5	1.9	3.4	15		2.25	50	22	28	33	40	73	25	19	A-7-6	17			Mc			
		SS-3	4.5	6.0	3.4	4.9	10		1.75							27	18	A-7-6	16						
		SS-4	6.0	7.5	4.9	6.4	12		1.5							22	18	A-7-6	16						
2	B 019-0 21	SS-1	1.5	3.0	-0.7	0.8	19	15	4.5	31	17	14	36	25	61	13	14	A-6a	7	633					
		SS-2	3.0	4.5	0.8	2.3	19		4.5	28	17	11	38	26	64	14	14	A-6a	6						
		SS-3	4.5	6.0	2.3	3.8	15		4.5							16	14	A-6a	10						
		SS-4	6.0	7.5	3.8	5.3	15		4.5							15	14	A-6a	10						
3	B 020-0 21	SS-1	1.5	3.0	-0.5	1.0	29	10	4.5	26	17	9	35	25	60	12	12	A-4a	5	393					
		SS-2	3.0	4.5	1.0	2.5	23		4.5	27	16	11	34	25	59	12	14	A-6a	5						
		SS-3	4.5	6.0	2.5	4.0	10		3.75							13	16	A-6b	16						
		SS-4	6.0	7.5	4.0	5.5	11		2.25							14	16	A-6b	16						
4	B 021-0 21	SS-1	1.5	3.0	-0.4	1.1	20	16	4.5	32	18	14	36	30	66	13	14	A-6a	8	293					
		SS-2	3.0	4.5	1.1	2.6	23		4.5	29	18	11	37	28	65	14	14	A-6a	6						
		SS-3	4.5	6.0	2.6	4.1	20		4.5							15	14	A-6a	10						
		SS-4	6.0	7.5	4.1	5.6	16		4.5							15	14	A-6a	10						
5	B 022-0 21	SS-1	1.5	3.0	0.2	1.8	16	10								6	6	A-1-b	0						
		SS-2	3.0	4.5	1.8	3.3	12			NP	NP	NP	14	4	18	6	6	A-1-b	0						
		SS-3	4.5	6.0	3.3	4.8	11		4.25	38	19	19	39	36	75	18	16	A-6b	12	240					
		SS-4	6.0	7.5	4.8	6.3	10									11	6	A-1-a	0						
6	B 023-0 21	SS-1	1.5	3.0	0.3	1.8	20	18	4.5							14	16	A-6b	16	533					
		SS-2	3.0	4.5	1.8	3.3	18		4.5	35	19	16	31	27	58	16	16	A-6b	7						
		SS-3	4.5	6.0	3.3	4.8	30		4.25							11	16	A-6b	16						
		SS-4	6.0	7.5	4.8	6.3	34			32	18	14	22	16	38	9	14	A-6a	2						
7	B 024-0 21	SS-1	1.5	3.0	0.2	1.7	22	22	4.5							16	16	A-6b	16						
		SS-2	3.0	4.5	1.7	3.2	26		4.5	26	16	10	37	25	62	11	11	A-4a	5	1967					
		SS-3	4.5	6.0	3.2	4.7	29		4.5	25	16	9	38	22	60	10	11	A-4a	5						
		SS-4	6.0	7.5	4.7	6.2	35		4.5							10	10	A-4a	8						

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 7

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	12"

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	6
-------------------	----------

% Samples within 6 feet of subgrade			
N₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N₆₀< 12	18%	0.5 < HP ≤ 1	0%
12 ≤ N₆₀< 15	7%	1 < HP ≤ 2	7%
N₆₀≥ 20	46%	HP > 2	79%
M+	4%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

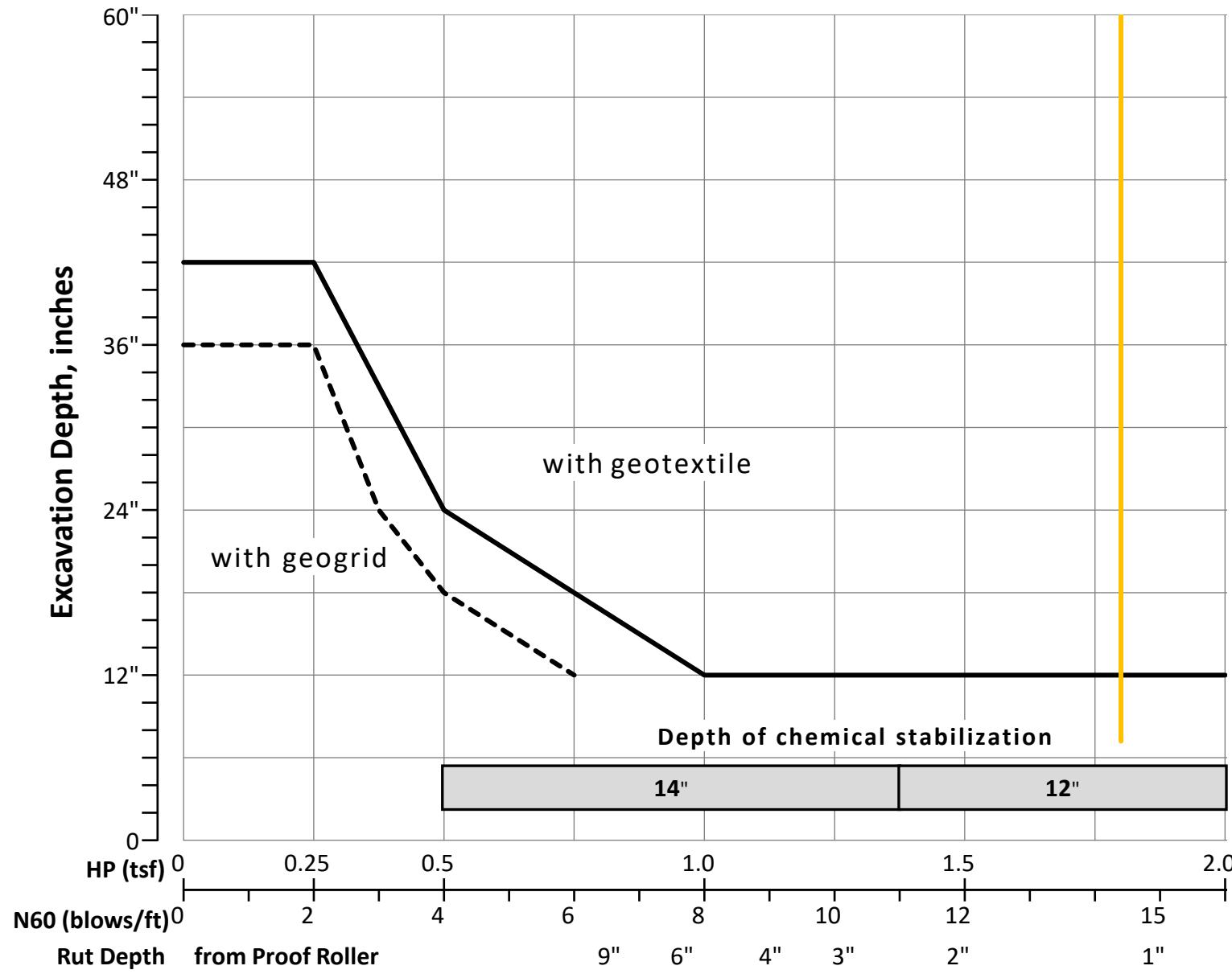
% Proposed Subgrade Surface	
Unstable & Unsuitable	6%
Unstable	6%
Unsuitable	0%

	N₆₀	N_{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M_c	M_{opt}	GI
Average	19	14	4.02	32	18	14	33	25	58	14	14	9
Maximum	35	22	4.50	50	22	28	39	40	75	27	19	17
Minimum	10	10	1.50	25	16	9	14	4	18	6	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	1	2	0	0	0	0	0	0	4	0	0	10	8	0	3	0	0	28
Percent	0%	4%	7%	0%	0%	0%	0%	0%	0%	14%	0%	0%	36%	29%	0%	11%	0%	0%	100%
% Rock Granular Cohesive	0%	25%										75%						100%	
Surface Class Count	0	0	2	0	0	0	0	0	0	2	0	0	7	5	0	1	0	0	17
Surface Class Percent	0%	0%	12%	0%	0%	0%	0%	0%	0%	12%	0%	0%	41%	29%	0%	6%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.02	0.50	<input type="checkbox"/> HP
14.43	6.00	<input type="checkbox"/> N60L

Average HP
Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****Ramp N****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 7, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**4**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-042-0-21	Ramp N	141+47	31	RT	CME 55T	63	910.3	909.6	0.7 C
2	B-004-0-21	Ramp N	142+87	123	LT	CME 45B	82	911.1	908.4	2.7 C
3	B-043-0-21	Ramp N	145+00	11	RT	CME 55T	63	906.0	906.2	0.2 F
4	B-005-0-21	Ramp N	146+30	42	LT	CME 45B	82	908.2	905.7	2.6 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable	
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable	
1	B 042-0 21	SS-1	1.5	3.0	0.8	2.3	20	8	4.5	30	17	13	37	28	65	14	14	A-2-4	0	1280					
		SS-2	3.0	4.5	2.3	3.8	16		2.75							22	14	A-6a	10						
		SS-3	4.5	6.0	3.8	5.3	8		3.5	53	25	28	40	49	89	28	22	A-7-6							
		SS-4	6.0	7.5	5.3	6.8	18																		
2	B 004-0 21	SS-1	1.5	3.0	-1.2	0.3	60	25	4.5	34	19	15	40	33	73	17	14	A-1-b	0						
		SS-2	3.0	4.5	0.3	1.8	35		3.5	32	17	15	36	32	68	16	14	A-6a	10	20		Mc			
		SS-3	4.5	6.0	1.8	3.3	29		3.5							24	18	A-7-6	16						
		SS-4	6.0	7.5	3.3	4.8	25																		
3	B 043-0 21	SS-1	1.5	3.0	1.7	3.2	27	17	4.5	28	17	11	39	27	66	14	14	A-6a	7	160					
		SS-2	3.0	4.5	3.2	4.7	22		4.5							13	14	A-6a	10						
		SS-3	4.5	6.0	4.7	6.2	17		4.5	25	18	7	32	21	53	12	13	A-4a	4						
		SS-4	6.0	7.5	6.2	7.7	20		4.5							10	10	A-4a							
4	B 005-0 21	SS-1	1.5	3.0	-1.1	0.4	35	20	4.5	26	17	9	34	22	56	14	12	A-1-b	0						
		SS-2	3.0	4.5	0.4	1.9	23		4							15	10	A-4a	4	260					
		SS-3	4.5	6.0	1.9	3.4	20		4.25							12	10	A-4a	8			Mc			
		SS-4	6.0	7.5	3.4	4.9	23																		

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 4

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	NA

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	0"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	7
-------------------	----------

% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ < 12	7%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	0%	1 < HP ≤ 2	0%
N ₆₀ ≥ 20	73%	HP > 2	80%
M+	13%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

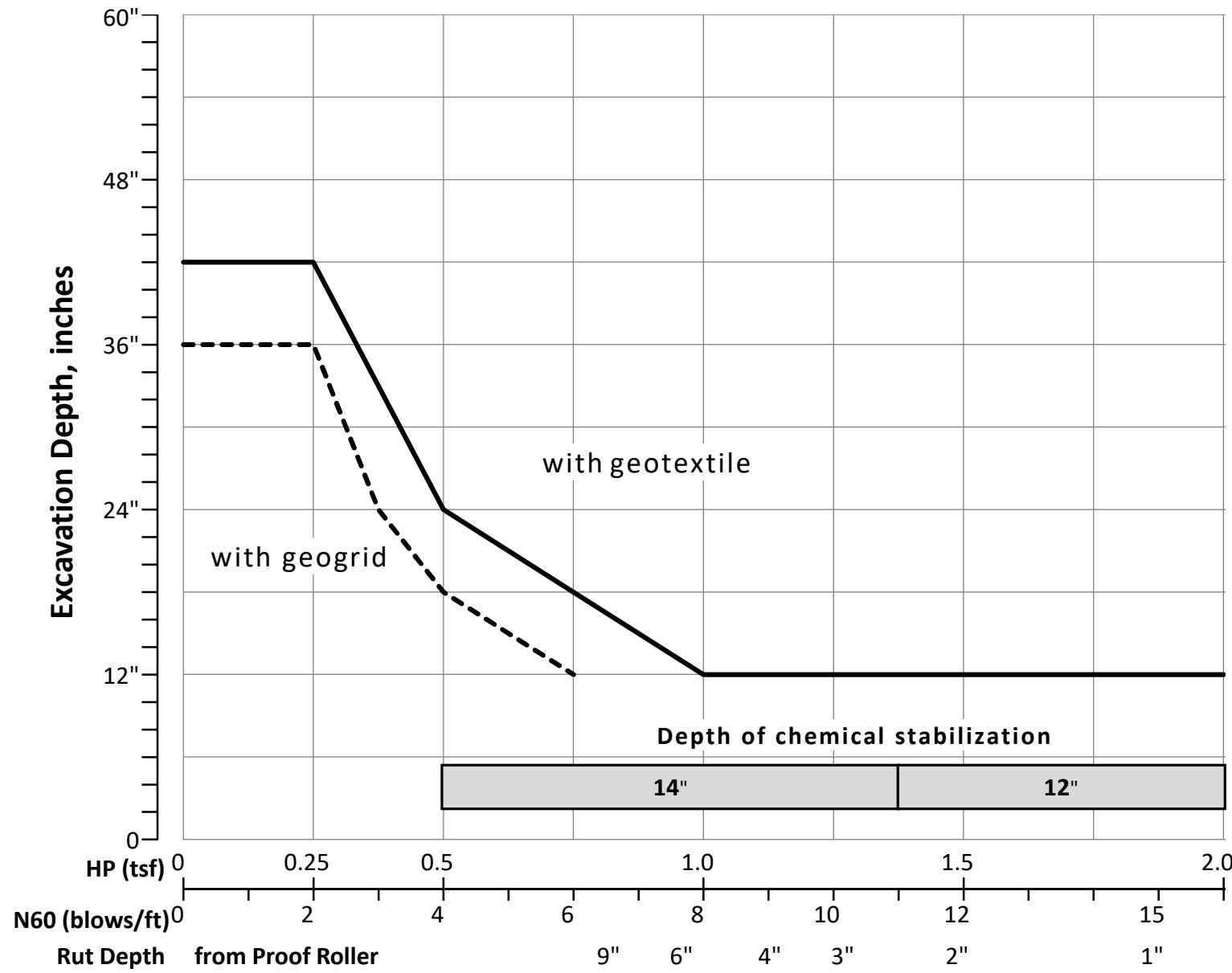
% Proposed Subgrade Surface	
Unstable & Unsuitable	22%
Unstable	22%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	25	18	4.08	33	19	14	37	30	67	15	13	7
Maximum	60	25	4.50	53	25	28	40	49	89	28	22	16
Minimum	8	8	2.75	25	17	7	32	21	53	7	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	2	1	0	0	0	0	0	5	0	0	6	0	0	2	0	0	16
Percent	0%	0%	13%	6%	0%	0%	0%	0%	0%	31%	0%	0%	38%	0%	0%	13%	0%	0%	100%
% Rock Granular Cohesive	0%	50%										50%						100%	
Surface Class Count	0	0	2	1	0	0	0	0	0	2	0	0	4	0	0	0	0	9	
Surface Class Percent	0%	0%	22%	11%	0%	0%	0%	0%	0%	22%	0%	0%	44%	0%	0%	0%	0%	100%	



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.08	0.50	<input type="checkbox"/> HP
17.50	6.00	<input type="checkbox"/> N60L

Average HP
Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only.

(Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

FRA-071/270-28.27/25.99A**105435****Ramp O****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 7, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**5**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-036-0-21	Ramp O	19+28	29	RT	CME 75T	79	916.2	914.5	1.7 C
2	B-037-0-21	Ramp O	23+07	2	RT	CME 75T	79	926.4	925.7	0.7 C
3	B-039-0-21	Ramp O	31+80	5	LT	CME 75T	79	929.8	928.8	1.0 C
4	B-040-0-21	Ramp O	34+43	20	RT	CME 55T	63	925.8	924.7	1.1 C
5	B-041-0-21	Ramp O	37+47	58	LT	CME 55T	63	921.7	921.3	0.4 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)	
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable		
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable		
1	B 036-0 21	SS-1	1.5	3.0	-0.2	1.3	14	8	4.5	24	14	10	34	23	57	10	10	A-4a	4	600						
		SS-2	3.0	4.5	1.3	2.8	13		4.5	23	14	9	38	24	62	12	10	A-4a	5							
		SS-3	4.5	6.0	2.8	4.3	16		4.5							11	10	A-4a	8							
		SS-4	6.0	7.5	4.3	5.8	8		4							11	10	A-4a	8							
2	B 037-0 21	SS-1	1.5	3.0	0.8	2.4	18	18	4.5	23	14	9	39	25	64	10	10	A-4a	6	780						
		SS-2	3.0	4.5	2.4	3.9	20		4.5	23	13	10	36	24	60	9	10	A-4a	5							
		SS-3	4.5	6.0	3.9	5.4	25		4.5							10	10	A-4a	8							
		SS-4	6.0	7.5	5.4	6.9	38		4.5							9	10	A-4a								
3	B 039-0 21	SS-1	1.5	3.0	0.5	2.0	18	14	4.5	27	16	11	34	28	62	12	14	A-6a	6	400						
		SS-2	3.0	4.5	2.0	3.5	16		4.5							12	14	A-6a	10							
		SS-3	4.5	6.0	3.5	5.0	14		4.5							13	14	A-6a	10							
		SS-4	6.0	7.5	5.0	6.5	16		4.5	27	14	13	35	27	62	11	14	A-6a								
4	B 040-0 21	SS-1	1.5	3.0	0.4	1.9	26	26	4.5	23	14	9	35	25	60	8	10	A-4a	5	6033						
		SS-2	3.0	4.5	1.9	3.4	33		4.5	27	16	11	37	28	65	11	14	A-6a	6							
		SS-3	4.5	6.0	3.4	4.9	29		4.5							10	14	A-6a	10							
		SS-4	6.0	7.5	4.9	6.4	33		4.5							13	14	A-6a	10							
5	B 041-0 21	SS-1	1.5	3.0	1.1	2.6	11	11	4.5	25	15	10	38	27	65	12	10	A-4a	6	2633						
		SS-2	3.0	4.5	2.6	4.1	19		4.5	32	17	15	36	33	69	13	14	A-6a	9							
		SS-3	4.5	6.0	4.1	5.6	19		4.5							14	14	A-6a	10							
		SS-4	6.0	7.5	5.6	7.1	27		4.5							13	14	A-6a								

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 5

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	12"

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	7
------------	---

% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ < 12	10%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	15%	1 < HP ≤ 2	0%
N ₆₀ ≥ 20	40%	HP > 2	100%
M+	0%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

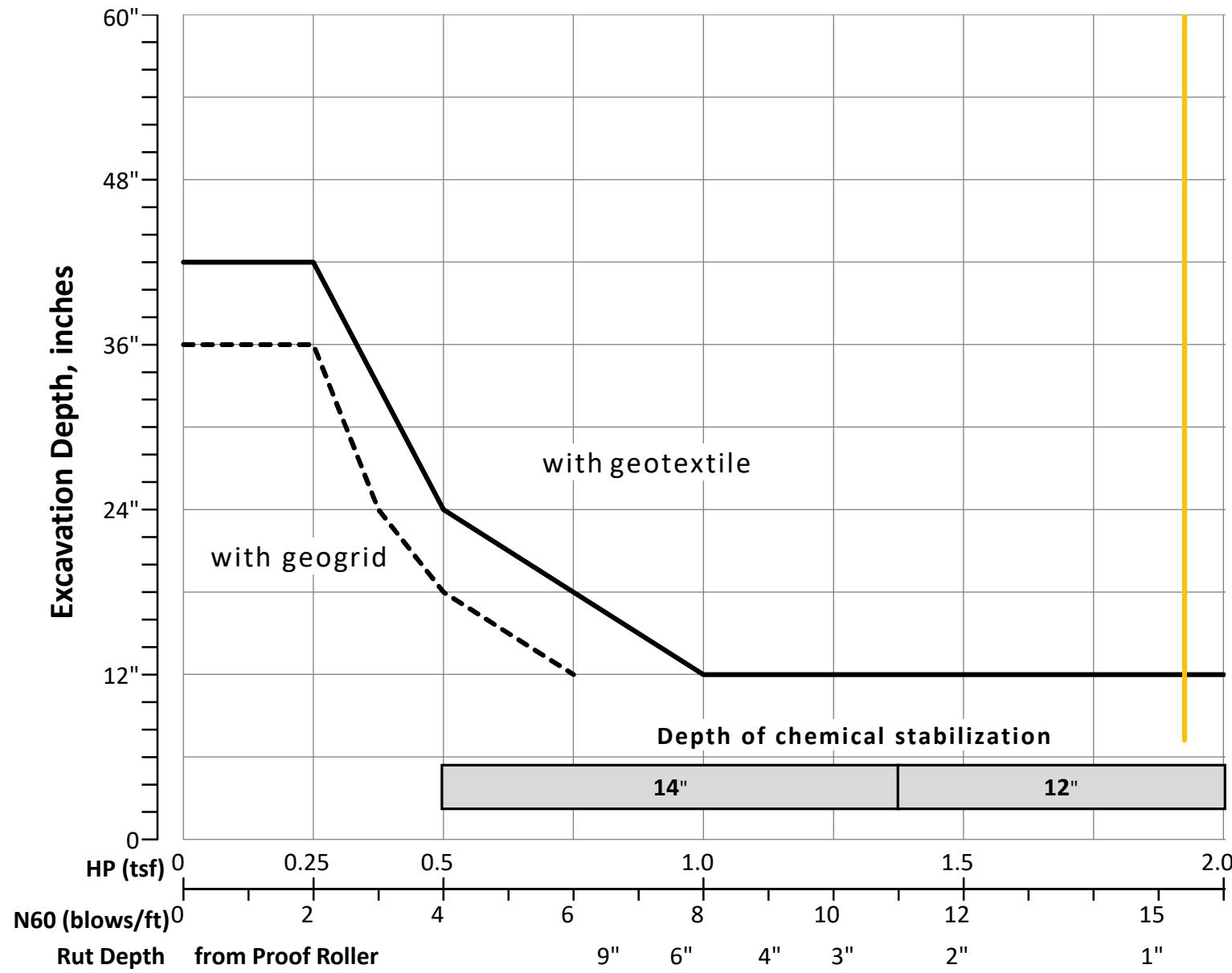
% Proposed Subgrade Surface	
Unstable & Unsuitable	10%
Unstable	10%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	21	15	4.48	25	15	11	36	26	63	11	12	7
Maximum	38	26	4.50	32	17	15	39	33	69	14	14	10
Minimum	8	8	4.00	23	13	9	34	23	57	8	10	4

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	0	0	0	0	0	0	0	10	0	0	10	0	0	0	0	20	
Percent	0%	0%	0%	0%	0%	0%	0%	0%	0%	50%	0%	0%	50%	0%	0%	0%	0%	100%	
% Rock Granular Cohesive	0%																	100%	
Surface Class Count	0	0	0	0	0	0	0	0	0	6	0	0	4	0	0	0	0	10	
Surface Class Percent	0%	0%	0%	0%	0%	0%	0%	0%	0%	60%	0%	0%	40%	0%	0%	0%	0%	100%	



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.48	0.50	<input type="checkbox"/> HP
15.40	6.00	<input type="checkbox"/> N60L

Average HP

Average N_{60L}

OHIO DEPARTMENT OF TRANSPORTATION**OFFICE OF GEOTECHNICAL ENGINEERING****PLAN SUBGRADES**
Geotechnical Bulletin GB1

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FRA-071/270-28.27/25.99A**105435****Ramp P****NEAS, Inc.**

Prepared By: MWJ
Date prepared: Wednesday, September 07, 2022

Chunmei (Melinda) He, Ph.D, P.E.
2800 Corporate Exchange Drive
Suite 240
Columbus, OH, 43231
614-714-0299
che@neasinc.com

NO. OF BORINGS:**7**

#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-022-0-21	Ramp P	1000+06	26	RT	CME 45B	82	920.6	919.7	0.9 C
2	B-023-0-21	Ramp P	1003+80	22	LT	CME 45B	82	923.6	923.0	0.6 C
3	B-024-0-21	Ramp P	1007+63	6	RT	CME 45B	82	925.6	926.2	0.6 F
4	B-025-0-21	Ramp P	1011+38	22	LT	CME 45B	82	935.5	935.2	0.3 C
5	B-026-0-21	Ramp P	1018+46	4	RT	CME 45B	82	928.1	926.6	1.4 C
6	B-027-0-21	Ramp P	1022+29	18	LT	CME 45B	82	913.7	911.1	2.6 C
7	B-001-0-21	Ramp P	1024+89	27	LT	CME 45B	82	913.0	910.7	2.3 C



#	Boring	Sample	Sample Depth		Subgrade Depth		Standard Penetration		HP (tsf)	Physical Characteristics						Moisture		Ohio DOT		Sulfate Content (ppm)	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in inches)	
			From	To	From	To	N ₆₀	N _{60L}		LL	PL	PI	% Silt	% Clay	P200	M _c	M _{opt}	Class	GI		Unsuitable	Unstable	Unsuitable	Unstable		
			From	To	From	To	16	10		NP	NP	NP	14	4	18	6	6	A-1-b	0							
1	B 022-0 21	SS-1	1.5	3.0	0.6	2.1	16	4.25	38	19	19	39	36	75	18	6	A-1-b	0	12"							
		SS-2	3.0	4.5	2.1	3.6	12																			
		SS-3	4.5	6.0	3.6	5.1	11																			
		SS-4	6.0	7.5	5.1	6.6	10																			
2	B 023-0 21	SS-1	1.5	3.0	0.9	2.4	20	18	4.5											12"						
		SS-2	3.0	4.5	2.4	3.9	18		4.5	35	19	16	31	27	58	16	16	A-6b	7							
		SS-3	4.5	6.0	3.9	5.4	30		4.25																	
		SS-4	6.0	7.5	5.4	6.9	34			32	18	14	22	16	38	9	14	A-6a								
3	B 024-0 21	SS-1	1.5	3.0	2.1	3.6	22	22	4.5											12"						
		SS-2	3.0	4.5	3.6	5.1	26		4.5	26	16	10	37	25	62	11	11	A-4a	5							
		SS-3	4.5	6.0	5.1	6.6	29		4.5	25	16	9	38	22	60	10	11	A-4a								
		SS-4	6.0	7.5	6.6	8.1	35		4.5																	
4	B 025-0 21	SS-1	1.5	3.0	1.2	2.7	14	5	4.5										12"							
		SS-2	3.0	4.5	2.7	4.2	5		2.5	25	15	10	39	23	62	13	10	A-4a	5							
		SS-3	4.5	6.0	4.2	5.7	8		3.75	24	15	9	38	24	62	13	10	A-4a	5							
		SS-4	6.0	7.5	5.7	7.2	10		4																	
5	B 026-0 21	SS-1	1.5	3.0	0.1	1.6	8	5	3	32	17	15	37	32	69	17	14	A-6a	9	673					12"	12"
		SS-2	3.0	4.5	1.6	3.1	10		3.75	33	19	14	38	33	71	19	14	A-6a	9		N ₆₀ & Mc					
		SS-3	4.5	6.0	3.1	4.6	5		3.5											13	14	A-6a	10			
		SS-4	6.0	7.5	4.6	6.1	5		2.5											17	14	A-6a	10			
6	B 027-0 21	SS-1	1.5	3.0	-1.1	0.4	7	7		NP	NP	NP	19	14	33	10	10	A-2-4	0						15"	15"
		SS-2	3.0	4.5	0.4	1.9	19			34	20	14	27	21	48	12	15	A-6a	4	807						
		SS-3	4.5	6.0	1.9	3.4	18													16	14	A-6a	10			
		SS-4	6.0	7.5	3.4	4.9	18													14	14	A-6a	10			
7	B 001-0 21	SS-1	1.5	3.0	-0.8	0.7	19	19		NP	NP	NP	24	15	39	10	11	A-4a	1							15"
		SS-2	3.0	4.5	0.7	2.2	20		4.5	39	19	20	43	41	84	21	16	A-6b	12	233						
		SS-3	4.5	6.0	2.2	3.7	29		4.5											18	16	A-6b	16			
		SS-4	6.0	7.5	3.7	5.2	26		3.75											25	16	A-6b	16			

PID: 105435

County-Route-Section: FRA-071/270-28.27/25.99A

No. of Borings: 7

Geotechnical Consultant: NEAS, Inc.

Prepared By: MWJ

Date prepared: 9/7/2022

Chemical Stabilization Options		
320	Rubblize & Roll	Option
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	12"

Excavate and Replace Stabilization Options	
Global Geotextile Average(N60L):	12"
Average(HP):	0"
Global Geogrid Average(N60L):	0"
Average(HP):	0"

Design CBR	6
-------------------	----------

% Samples within 6 feet of subgrade			
N ₆₀ ≤ 5	11%	HP ≤ 0.5	0%
N ₆₀ < 12	37%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	7%	1 < HP ≤ 2	0%
N ₆₀ ≥ 20	33%	HP > 2	67%
M+	11%		
Rock	0%		
Unsuitable	0%		

Excavate and Replace at Surface	
Average	0"
Maximum	0"
Minimum	0"

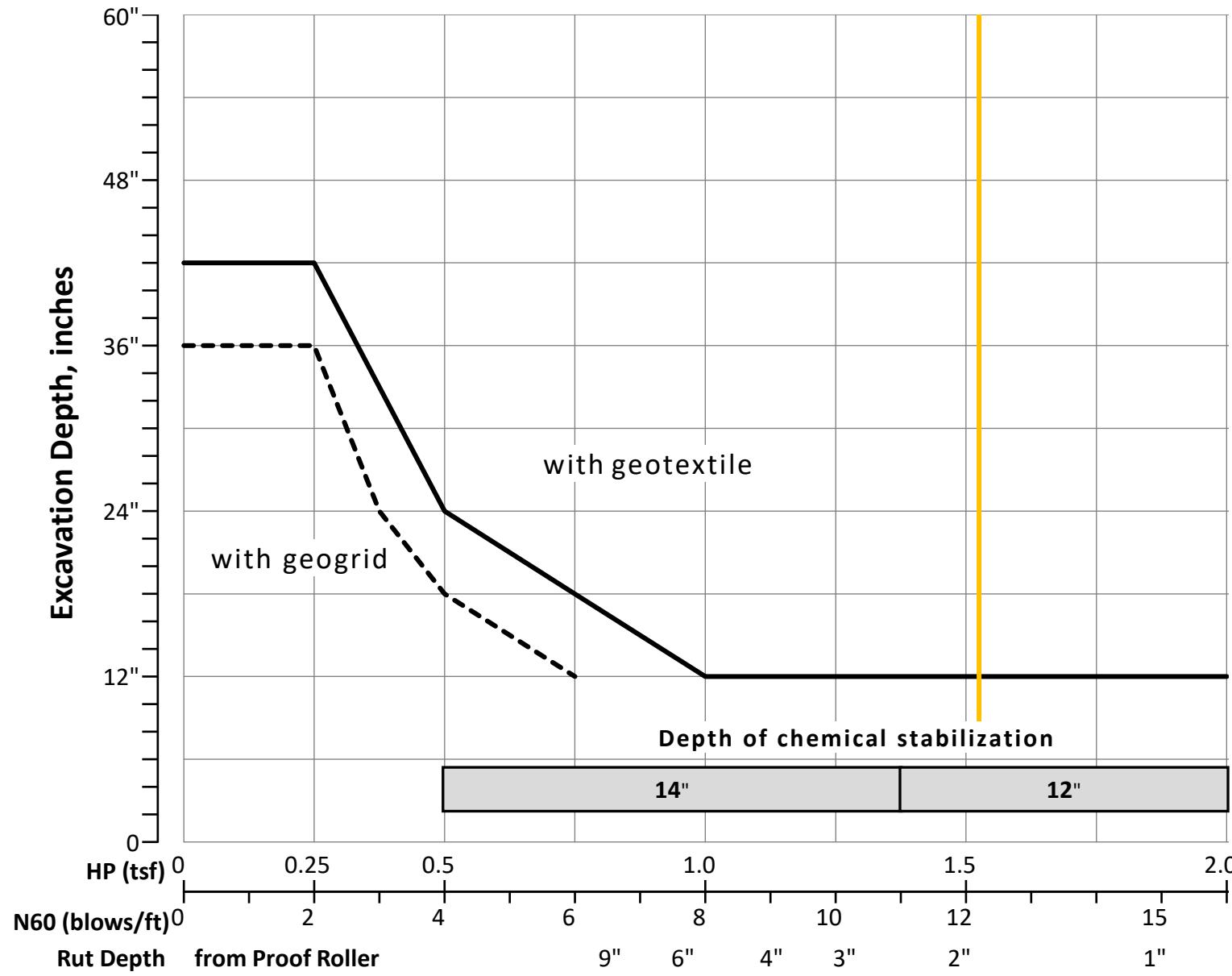
% Proposed Subgrade Surface	
Unstable & Unsuitable	27%
Unstable	27%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	M _c	M _{opt}	GI
Average	17	12	3.99	31	18	14	32	24	56	14	13	9
Maximum	35	22	4.50	39	20	20	43	41	84	25	16	16
Minimum	5	5	2.50	24	15	9	14	4	18	6	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	1	2	1	0	0	0	0	0	8	0	0	8	8	0	0	0	0	28
Percent	0%	4%	7%	4%	0%	0%	0%	0%	0%	29%	0%	0%	29%	29%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%	43%										57%						100%	
Surface Class Count	0	0	2	1	0	0	0	0	0	3	0	0	4	5	0	0	0	0	15
Surface Class Percent	0%	0%	13%	7%	0%	0%	0%	0%	0%	20%	0%	0%	27%	33%	0%	0%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization

OVERRIDE TABLE

Calculated Average	New Values	Check to Override
3.99	0.50	<input type="checkbox"/> HP
12.29	6.00	<input type="checkbox"/> N60L

Average HP
Average N_{60L}