CTL Engineering, Inc.

2860 Fisher Road, P.O. Box 44548, Columbus, Ohio 43204-3538

Phone: 614/276-8123; Fax: 614/276-6377

Email: ctl@ctleng.com

AN EMPLOYEE OWNED COMPANY

ENGINEERING S

Consulting Engineers • Testing • Inspection Services • Analytical Laboratories

Established 1927

November 6, 2023

EMH&T 5500 New Albany Road Columbus, Ohio 43054

Attention: Mr. Mark Rahall, P.E

Associate, Project Manager

Reference: Roadway Subsurface Exploration- Final Report

Broad Street and Hamilton Road Intersection Improvements

FRA-16-6.87, PID No. 105768

Franklin County, Ohio

CTL Project No. 20050033COL

Dear Mr. Rahall:

CTL Engineering, Inc. has completed the Roadway Exploration for this project. Enclosed is the digital (pdf) copy of the Final Report.

Thank you for the opportunity to work with you on this project. If you have any questions or need further information, please feel free to contact our office.

Respectfully Submitted

CTL ENGINEERING, INC.

Joe Grani, P.E Project Engineer

ROADWAY SUBSURFACE EXPLORATION-FINAL REPORT

BROAD STREET AND HAMILTON ROAD INTERSECTION IMPROVEMENTS FRA-16-6.87, PID NO. 105768 FRANKLIN COUNTY, OHIO CTL PROJECT NO. 20050033COL

PREPARED FOR:

EMH&T, INC. 5500 NEW ALBANY ROAD COLUMBUS, OHIO 43054

PREPARED BY:

CTL ENGINEERING, INC. 2860 FISHER ROAD COLUMBUS, OHIO 43204 Phone 614-276-8123 Fax 614-276-6377

November 6, 2023



TABLE OF CONTENTS

		PAGE
I.	EXECUTIVE SUMMARY	1
II.	INTRODUCTION	1
III.	GEOLOGY AND OBSERVATIONS OF THE PROJECT	2
IV.	EXPLORATION	3
V.	FINDINGS	3
VI.	ANALYSES AND RECOMMENDATIONS	4
	A. Subgrade Considerations	5
	B. General Construction and Earthwork	6
VII.	CHANGED CONDITIONS	6
VIII.	TESTING AND OBSERVATION	7
IX.	CLOSING	7
APPE	ENDIX A ROADWAY EXPLORATION SHEETS ENDIX B TEST BORING RECORDS ENDIX C PAVEMENT CORE REPORT	
	ENDIX C PAVEMENT CORE REPORT ENDIX D GB1 ANALYSIS	



Client: EMH&T CTL Project No. 20050033COL November 6, 2023 Page 1

I. <u>EXECUTIVE SUMMARY</u>

The project involves performing improvements to the intersection of State Route (SR) 16 (East Broad Street) and SR 317 (Hamilton Road) in Franklin County, Ohio.

Twelve (12) soil test borings, identified as B-001-0-19 through B-012-0-19, were drilled to depths ranging from 5.5 to 8.0 feet below grade. Auger refusal was encountered on concrete in boring B-008-0-19 at a depth of 5.5 feet below grade. Twelve (12) pavement cores were also performed at the boring locations. Two (2) additional pavement cores, designated as B-008-1-19 and B-008-2-19, were performed offset from the location of a failed joint at the composite pavement section of SR 317.

Below the surface cover, the borings encountered fine-grained cohesive and coarse-grained granular soils extending down to the test boring termination depths or auger refusal depth in boring B-008-0-19. The cohesive soils were classified as sandy silt (A-4a), silt and clay (A-6a), silty clay (A-6b), elastic clay (A-7-5), or clay (A-7-6). Granular soils were described as gravel and/or stone fragments with sand (A-1-b), gravel and/or stone fragments with sand and silt (A-2-4), gravel and/or stone fragments with sand, silt and clay (A-2-6), fine sand (A-3) or coarse and fine sand (A-3a).

According to requirements outlined in ODOT's Geotechnical Bulletin 1 (GB1), some of the subgrade soils will require stabilization. Please refer to the *Analyses and Recommendation* section of this report for additional details.

Unsuitable soils (A-7-5) were encountered along SR 317 in boring B-007-0-19, near Station 269+5.7, between approximate depths of 1.0 and 2.5 feet below the proposed subgrade level. These A-7-5 soils are not suitable to be left in place in the upper 36 inches of proposed subgrade. No roadway widening is proposed near boring B-007-0-19. However, if full depth replacement of the existing pavement is required in this area, and the pavement subgrade is exposed, then the unsuitable A-7-5 soils will need to be removed to a depth of 36 inches below the top of the proposed pavement subgrade.

Based on the subsurface conditions encountered in the borings, and the results of GB1 analyses, the pavement for this project may be designed using a CBR value of 6.0.

II. <u>INTRODUCTION</u>

The project involves performing improvements to the intersection of State Route (SR) 16 (East Broad Street) and SR 317 (Hamilton Road) in Franklin County, Ohio. The improvements will involve pavement widening for SR 16 and SR 317.

The project limits and approximate cut/fill depths were taken from the plans provided by EMH&T personnel, and are shown in Table 1 below:



Client: EMH&T

CTL Project No. 20050033COL

November 6, 2023

Page 2

Table 1. Project/Work Limits and Cut/Fill Depths

Roadway	Approximate Begin Project Station	Approximate End Project Station	Maximum Cut (feet)	Maximum Fill (feet)
SR 16	360+72.24	377+27.30	NA	NA
SR 317	269+10.11	288+60.17	NA	NA

This report is a Final Roadway Exploration report.

III. GEOLOGY AND OBSERVATIONS OF THE PROJECT

According to the Ohio Department of Natural Resources (ODNR), *Physiographic Regions of Ohio*, the site lies in the Columbus Low Land region of the Central Low Land Till Plains section of Ohio.

According to Bedrock Geologic Map of Ohio, the bedrock below the site consists of Devonian age shale of the Ohio Shale Formation.

According to web based mapping from United States Department of Agriculture, Natural Resources Conservation Service, the project area contains primarily Bennington-Urban land complex, 0 to 2 percent slopes (BfA) and Bennington-Urban land complex, 0 to 6 percent slopes (BfB). According to the *Soil Survey of Franklin County, Ohio*, these soils are considered somewhat poorly drained and exhibit moderately low to moderately high permeability.

According to the ODNR's Mines of Ohio website, no mines were located within the project area. According to information obtained from ODNR's Karst Area Map, the project lies in the area that is not known to contain karst features.

The most recent site visit by CTL personnel was performed on August 19, 2020. Some longitudinal and transverse cracks were noted on the pavement of SR 16 and SR 317. Alligator cracking was noted on SR 317, between approximate Stations 271+00 and 272+00. Several underground and overhead utility lines were present within the project limits.

Historic geotechnical records were searched for on the ODOT TIMS website. Historic auger probe sampling records were found within the project area. However, no historic test boring records were found within the project area.



Client: EMH&T CTL Project No. 20050033COL November 6, 2023 Page 3

IV. <u>EXPLORATION</u>

Twelve (12) soil test borings, identified as B-001-0-19 through B-012-0-19, were drilled to depths ranging from 5.5 to 8.0 feet below grade. Auger refusal was encountered on concrete in boring B-008-0-19 at a depth of 5.5 feet below grade. Twelve (12) pavement cores were also performed at the boring locations. Two (2) additional pavement cores, designated as B-008-1-19 and B-008-2-19, were performed offset from the location of a failed joint at the composite pavement section of SR 317. The borings were drilled at the approximate locations indicated on the Soil Profile sheets provided in Appendix A. Survey data at the test boring locations were provided by personnel from EMH&T.

The borings were performed with a track-mounted drill rig utilizing hollow stem augers (HSA), between August 18 and August 20, 2020. Standard penetration tests were conducted using a 140-pound automatic hammer falling 30 inches to drive 2-inch O.D. split barrel samplers. The energy transfer ratio associated with the automatic SPT hammer was 89.3 percent. The hammer was calibrated on August 5, 2020.

Soil samples obtained from drilling operation, were preserved in glass jars, visually classified in the field and laboratory, and tested for natural moisture content. Representative soil samples were subjected to laboratory testing including grain size distribution, Atterberg limits and hand penetrometer.

V. FINDINGS

A summary of the pavement thickness encountered in the test borings and the pavement cores is provided in Table 2 below. Please refer to Appendix C Pavement Core Report for details on pavement cores.



Client: EMH&T

CTL Project No. 20050033COL

November 6, 2023

Page 4

Table 2: Existing Pavement Thickness

		ing ravelment rin		
Roadway	Doring No.	Material Thicknes	s (inches)	
	Boring No.	Asphalt	Concrete	Granular Base
	B-001-0-19	15		3
	B-002-0-19	12		
SR 16	B-003-0-19	10		4
SK 10	B-004-0-19	11		9
	B-005-0-19	9		3
	B-006-0-19	11		
	B-007-0-19	2	10.5	
	B-008-0-19	6	6.75	
	B-008-1-19	5	6	
SR 317	B-008-2-19	5	7	
SK 317	B-009-0-19	4	13	
	B-010-0-19	14		
	B-011-0-19	13		
	B-012-0-19	5	11	

Below the surface cover, borings B-003-0-19 through B-006-0-19, B-008-0-19, and B-012-0-19 encountered fill materials to depths ranging from 2.5 to 5.5 feet below grade. The fill materials were described as gravel and/or stone fragments with sand (A-1-b), gravel and/or stone fragments with sand and silt (A-2-4), gravel and/or stone fragments with sand, silt and clay (A-2-6), coarse and fine sand (A-3a), or silt and clay (A-6a). These soils exhibited standard penetration (N_{60}) values ranging from 10 to 73 blows per foot (bpf), with natural moisture content values ranging from 5 to 22 percent.

Below the fill materials or surface cover, the test borings, except for boring B-008-0-19, encountered both fine-grained cohesive and coarse-grained granular soils extending down to the test boring termination depths. The soils were classified as fine sand (A-3), sandy silt (A-4a), silt and clay (A-6a), silty clay (A-6b), elastic clay (A-7-5), or clay (A-7-6). These soils exhibited N₆₀ values ranging from 7 to 55 bpf, with natural moisture content values ranging from 10 to 32 percent. Auger refusal in concrete was encountered in boring B-008-0-19 at 5.5 feet below grade.

No groundwater was encountered in the test borings at any time during the field exploration. After removing the augers, no soil cave-in was observed in the test borings.

VI. ANALYSES AND RECOMMENDATIONS

Based on the soil and rock data obtained from the field and laboratory testing, the following recommendations are provided.



Client: EMH&T

CTL Project No. 20050033COL

November 6, 2023

Page 5

A. Subgrade Considerations

A subgrade analysis was performed utilizing the subsurface information from the drilled borings along with ODOT Geotechnical Bulletin 1 (GB1) guidelines. A copy of the GB1 spreadsheet is provided in Appendix D. For estimating cut/fill per GB1, the proposed pavement thickness was taken as 1.5 feet.

The natural moisture content values of the near surface soil samples ranged from 5 to 32 percent, averaging 18 percent. The estimated optimum moisture content (OMC) values ranged from 0 to 25 percent, averaging 15 percent. On average, the natural moisture content values were 3 percent higher than the optimum moisture content values.

Based on the requirements outlined in GB1, it is estimated that subgrade stabilization will be required in some areas within the project limits. The subgrade stabilization may consist of excavate and replace per Item 204. The approximate areas and depths are summarized in Table 3. The approximate depth of excavate and replace is measured from the top of the proposed pavement subgrade level. The locations and values are only an estimate. The actual depths and horizontal limits of excavate and replace will be determined by the Project Engineer in the field based upon proofrolling.

Table 3: Estimated Excavate and Replace

Location	Approximate Limits	Approximate Depth of Excavate and Replace (inches)
SR 16	From Sta. 367+50 to Sta. 371+50	12
CD 217	From Sta. 271+00 to Sta. 279+25	12
SR 317	From Sta. 284+25 to Sta. 288+60.17	12

If the soils at the excavated depth exhibit unstable conditions, a bridge lift should be placed as outlined in Item 203.05 of the ODOT Construction and Material Specifications.

It is CTL's opinion that any subgrade stabilization in the proposed widening areas will most likely consist of excavate and replace at isolated locations. Therefore, no global stabilization information is included. It should be noted that chemical stabilization in the proposed widening areas may not be feasible due to space limitations.

Group Index values were calculated for each of the samples tested. The Group Index values for soils ranged from 0 to 20, averaging 10. This average Group



Client: EMH&T CTL Project No. 20050033COL November 6, 2023 Page 6

Index value corresponds to an estimated California Bearing Ratio (CBR) value of 6.0. The pavement for this project may be designed using a CBR value of 6.0.

It is CTL's opinion that mill and overlay will generally be acceptable for the pavement resurfacing areas. However, alligator cracking was noted in an area between approximate Stations 271+00 and 272+00. Additionally, the concrete base in cores B-008-1-19 and B-008-2-19 (approx. Station 271+70) was broken/not intact. In this area, the pavement repair will likely need to consist of full depth replacement.

Unsuitable soils (A-7-5) were encountered along SR 317 in boring B-007-0-19, near Station 269+05.7 SR 317, between approximate depths of 1.0 and 2.5 feet below the proposed subgrade level. These A-7-5 soils are not suitable to be left in-place in the upper 36 inches of proposed subgrade. No roadway widening is proposed near boring B-007-0-19. However, if full depth replacement of the existing pavement is required in this area, and the pavement subgrade is exposed, then the unsuitable A-7-5 soils will need to be removed to a depth of 36 inches below the top of the proposed pavement subgrade.

B. General Construction and Earthwork

- 1. Site preparation and earthwork should be performed in accordance with the ODOT Construction and Material Specifications, and applicable Geotechnical Bulletins.
- 2. Temporary excavations in excess of 4 feet in depth should be sloped or shored according to OSHA requirements.

VII. CHANGED CONDITIONS

The evaluations, conclusions, and recommendations in this report are based on our interpretation of the field and laboratory data obtained during the exploration, our understanding of the project and our experience with similar sites and subsurface conditions using generally accepted geotechnical engineering practices. Although individual test borings are representative of the subsurface conditions at the boring locations on the dates drilled, they are not necessarily representative of the subsurface conditions between boring locations or subsurface conditions during other seasons of the year.

In the event that changes in the project are proposed, additional information becomes available, or if it is apparent that subsurface conditions are different from those provided in this report, CTL Engineering should be notified so that our recommendations can modified, if required



Client: EMH&T CTL Project No. 20050033COL November 6, 2023 Page 7

VIII. TESTING AND OBSERVATION

During the design process, it is recommended that CTL Engineering work with the project designers to confirm that the geotechnical recommendations are properly incorporated into the final plans and specifications, and to assist with establishing criteria for the construction observation and testing.

CTL Engineering is not responsible for independent conclusions, opinions and recommendations made by others based on the data and recommendations provided in this report. It is recommended that CTL be retained to provide construction quality control services on this project. If CTL Engineering is not retained for these services, CTL shall assume no responsibility for compliance with the design concepts or recommendations provided.

IX. CLOSING

The report was prepared by CTL Engineering, Inc. (Consultant) solely for the use of the Client in accordance with an executed contract. The Client's use of or reliance on this report is limited by the terms and conditions of the contract and by the qualifications and limitations stated in the report. It is also acknowledged that the Client's use of and reliance of this report is limited for reasons which include: actual site conditions that may change with time; hidden conditions, not discoverable within the scope of the assessment, may exist at the site; and the scope of the investigation may have been limited by time, budget and other constraints imposed by the Client.

Neither the report, nor its contents conclusions or recommendations, are intended for the use of any party other than the Client. Consultant and the Client assume no liability for any reliance placed on this report by such party. The rights of the Client under contract may not be assigned to any person or entity, without the consent of the Consultant which consent shall not be unreasonably withheld.

This geotechnical report does not address the environmental conditions of the site. The Consultant is not responsible for consequences or conditions arising from facts that were concealed, withheld, or not fully disclosed at the time the assessment was conducted.

To the fullest extent permitted by law, the Consultant and Client agree to indemnify and hold each other, and their officers and employees harmless from and against claims, damages, losses and expenses arising out of unknown or concealed conditions. Furthermore, neither the Consultant nor its employees shall be liable to the Owner in an amount in excess of the available professional liability insurance coverage of the Consultant. In addition, Client and Consultant agree neither shall be liable for any special, indirect or consequential damages of any kind or nature.



Client: EMH&T

CTL Project No. 20050033COL

November 6, 2023

Page 8

The Consultant's services have been provided consistent with its professional standard of care. No other warranties are made, either expressed or implied.

Respectfully Submitted,

CTL ENGINEERING, INC.

Joe Grani, P.E Project Engineer Shahedur Rahman Geotechnical Engineer

Caresta 6532005



APPENDIX A ROADWAY EXPLORATION SHEETS



HISTORIC RECORDS

HISTORIC GEOTECHNICAL RECORDS WERE SEARCHED FOR ON THE ODOT TIMS WEBSITE. HISTORIC GEOTECHNICAL RECORDS OF AUGER PROBE SAMPLING WERE FOUND WITHIN THE PROJECT AREA. HOWEVER, NO HISTORIC TEST BORING RECORDS WERE FOUND FOR THIS PROJECT.

GEOLOGY

 \bigcirc

ACCORDING TO THE OHIO DEPARTMENT OF NATURAL RESOURCES (ODNR), PHYSIOGRAPHIC REGIONS OF OHIO, THE SITE LIES IN THE COLUMBUS LOW LAND REGION OF THE CENTRAL LOW LAND TILL PLAINS SECTION OF OHIO.

ACCORDING TO BEDROCK GEOLOGIC MAP OF OHIO (2006), THE BEDROCK BELOW THE SITE CONSISTS OF DEVONIAN AGE SHALE OF OHIO SHALE FORMATION.

RECONNAISSANCE

THE MOST RECENT SITE VISIT WAS PERFORMED BY CTL PERSONNEL ON AUGUST 19, 2020 SOME LONGITUDINAL AND TRANSVERSE CRACKS WERE NOTED ON THE PAVEMENT OF SR 16 AND SR 317. ALLIGATOR CRACKING WAS NOTED ON SR 317, BETWEEN APPROXIMATE STATIONS 271+00 AND 272+00. SEVERAL UNDERGROUND AND OVERHEAD UTILITY LINES WERE PRESENT WITHIN THE PROJECT LIMITS.

SUBSURFACE EXPLORATION

TWELVE (12) TYPE A SOIL BORINGS, IDENTIFIED AS B-001-0-19 THROUGH B-012-0-19, WERE DRILLED TO DEPTHS RANGING FROM 5.5 TO 8.0 FEET BELOW GRADE. AUGER REFUSAL WAS ENCOUNTERED ON CONCRETE IN BORING B-008-0-19 AT A DEPTH OF 5.5 FEET BELOW GRADE. TWELVE (12) PAVEMENT CORES WERE ALSO PERFORMED AT THE BORING LOCATIONS. TWO (2) ADDITIONAL PAVEMENT CORES, DESIGNATED AS B-008-1-19 AND B-008-2-19, WERE PERFORMED OFFSET FROM THE LOCATION OF A FAILED JOINT AT THE COMPOSITE PAVEMENT SECTION OF SR 317. THE BORINGS WERE PERFORMED WITH A TRACK-MOUNTED DRILL RIG UTILIZING HOLLOW STEM AUGERS (HSA), BETWEEN AUGUST 18 AND AUGUST 20, 2020. STANDARD PENETRATION TESTS WERE CONDUCTED USING A 140-POUND AUTOMATIC HAMMER FALLING 30 INCHES TO DRIVE 2-INCH O.D. SPLIT BARREL SAMPLERS. THE ENERGY TRANSFER RATIO ASSOCIATED WITH THE AUTOMATIC SPT HAMMER WAS 89.3 PERCENT. THE HAMMER WAS CALIBRATED ON AUGUST 5, 2020.

EXPLORATION FINIDNGS

BELOW THE SURFACE COVER, BORINGS B-003-0-19 THROUGH B-006-0-19, B-008-0-19, AND B-012-0-19 ENCOUNTERED FILL MATERIALS TO DEPTHS RANGING FROM 2.5 TO 5.5 FEET BELOW GRADE. THE FILL MATERIALS WERE DESCRIBED AS GRAVEL AND/OR STONE FRAGMENTS WITH SAND (A-1-b). GRAVEL AND/OR STONE FRAGMENTS WITH SAND AND SILT (A-2-4), GRAVEL AND/OR STONE FRAGMENTS WITH SAND, SILT AND CLAY (A-2-6), COARSE AND FINE SAND (A-3a) OR SILT AND CLAY (A-6a). UNDERNEATH THE FILL MATERIALS OR SURFACE COVER, THE TEST BORINGS, EXCEPT FOR BORING B-008-0-19, ENCOUNTERED BOTH FINE-GRAINED COHESIVE AND COARSE-GRAINED GRANULAR SOILS EXTENDING DOWN TO THE TEST BORING TERMINATION DEPTHS. THE SOILS WERE CLASSIFIED AS FINE SAND (A-3), SANDY SILT (A-4a), SILT AND CLAY (A-6a), SILTY CLAY (A-6b), ELASTIC CLAY (A-7-5) OR CLAY (A-7-6). AUGER REFUSAL IN CONCRETE WAS ENCOUNTERED IN BORING B-008-0-19 AT 5.5 FEET BELOW GRADE.

NO GROUNDWATER WAS ENCOUNTERED IN THE TEST BORINGS AT ANY TIME DURING THE FIELD EXPLORATION. AFTER REMOVING THE AUGERS, NO SOIL CAVE-IN WAS OBSERVED IN THE TEST BORINGS.

SPECIFICATIONS

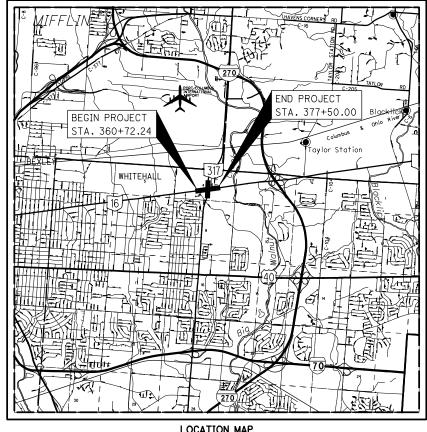
THIS GEOTECHNICAL EXPLORATION WAS PERFORMED IN ACCORDANCE WITH THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, OFFICE OF GEOTECHNICAL ENGINEERING, SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS, DATED JULY 2020.

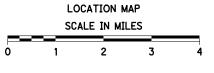
AVAILABLE INFORMATION

ALL AVAILABLE SOIL INFORMATION THAT CAN BE CONVINIENTLY SHOWN ON THE GEOTECHNICAL EXPLORATION SHEETS HAS BEEN REPORTED. ADDITIONAL SUBSURFACE EXPLORATIONS MAY HAVE BEEN MADE TO STUDY SOME SPECIAL ASPECT OF THE PROJECT. COPIES OF THIS DATA, IF ANY, MAY BE INSPECTED IN THE DISTRICT DEPUTY DIRECTOR'S OFFICE, THE OFFICE OF GEOTECHNICAL ENGINEERING AT 1980 WEST BROAD STREET.

	<u>LE</u>	GEND	ODOT	CL ACC	IEIED	
		DESCRIPTION	ODOT CLASS	CLASS MECH./		
		GRAVEL AND/OR STONE FRAGMENTS WITH SAND	A-1-b	1	1	
		FINE SAND	A-3	0	1	
		COARSE AND FINE SAND	A-3a	1	0	
		GRAVEL AND/OR STONE FRAGMENTS W/SAND AND SILT	A-2-4	3	0	
		GRAVEL AND/OR STONE FRAGMENTS W/SAND, SILT & CLAY	A-2-6	0	2	
)		SANDY SILT	A-4a	2	1	
		SILT AND CLAY	A-6a	8	4	
		SILTY CLAY	A-6b	1	9	
		ELASTIC CLAY	A-7-5	1	0	
)		CLAY	A-7-6	6	6	
5			TOTAL	23	24	
	I XXXXX	PAVEMENT OR BASE = X = APPROXIMATE THICKNESS	VISUAL			
		EXPLORATION LOCATION - PLAN VIEW				
		DRIVE SAMPLE AND/OR ROCK CORE BORING PLOTTED TO HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPHY		SCALE (ONLY.	
9	WC WC	INDICATES WATER CONTENT IN PERCENT.				
	WC					
	N ₆₀	INDICATES STANDARD PENETRATION RESISTANCE NORMALIZED TO 60% DRILL ROD ENERGY RATIO.				
	X/Y/D"	NUMBER OF BLOWS FOR STANDARD PENETRATION TEST (X= NUMBER OF BLOWS FOR 6 INCHES (UNCORRECTED). Y/D"= NUMBER OF BLOWS (UNCORRECTED) FOR D" OF PE		AT REFU	JSAL.	
	SS	INDICATES A SPLIT SPOON SAMPLE.				

INDEX OF SHEETS													
LOCA FROM STA.		PLAN VIEW SHEET	PROFILE SHEET	CUT MAX.	FILL EMB.								
SR 356+00 368+50	16 368+5Ø 380+00	3 4	3 4	- -	<1.0 FT <1.0 FT	<1.0 FT <1.0 FT							
SR 267+00 280+00	317 28Ø+ØØ 291+ØØ	5 6	5 6		<1.0 FT <1.0 FT	<1.0 FT <1.0 FT							







PARTICLE SIZE DEFINITIONS

12	2"	2.0	mm	0.42	mm	0.074	mm 0.00	5 mm
BOULDERS	COBBLES	GRAVEL	COARSE	SAND	FINE	SAND	SILT	CLAY
'	l	No. 10	SIEVE	No. 40	SIEVE	No. 200	SIEVE	

RECON. - SM 8/19/2020

DRILLING - CTL ENGINEERING INC 08/18/2020-08/20/2020

DRAWN - NKS 04/30/21 **REVIEWED -** JG 04/30/21

 ∞ 9 $\overline{}$ ⋖

 $\mathbf{\alpha}$

	\bigcirc	\bigcirc			\bigcirc	\bigcirc
D:\Drop Box\CII	2023\October\Dent	05\COL\Shahed\20050033C	OL ODOT \ Mod 31 10 23 \ 105768 ID001 dan Sheet	31-10-2023 13:50:12	ho	

%

REC

100

N60

27

SAMPLE

ID

SS-1

FROM TO

01.50-03.00

EXPLORATION NO.,

STATION & OFFSET

B-001-0-19

SUMMARY OF SOIL TEST DATA F BROAD STRFFT (SR 16)

GR

CS

13

FS

17

%

37

SILT CLAY

26

PL

21

LL

31

ΡĪ

10

WC

18

ODOT

CLASS (GI)

A-4a (6)

ppm

SO4

ΗP

tsf

4

LATITUDE = 39.97586 A-7-6 (18) 03.00-04.50 SS-2 10 89 2.75 2 9 36 52 54 25 29 23 LONGITUDE = -82.8771304.50-06.00 2.75 SAME AS SS-2 21 A-7-6 (VISUAL) SS-3 15 100 06.00-07.50 SS-4 10 56 0.5 SAME AS SS-2 32 A-7-6 (VISUAL) B-002-0-19 01.00-02.50 SS-1 22 67 28 9 30 24 32 20 12 18 A-6a (5) LATITUDE = 39.976075 02.50-04.00 SS-2 21 100 4.5 8 12 38 39 38 23 15 19 A-6a (10) LONGITUDE = -82.875601BROWN, SILTY CLAY, SOME SAND, LITTLE GRAVEL 04.00-05.50 SS-3 25 100 4.5 13 A-6b (VISUAL) 05.50-07.00 SS-4 15 100 3 SAME AS SS-3 19 A-6b (VISUAL) B-003-0-19 01.00-02.50 SS-1 30 56 35 21 12 11 22 18 4 12 A-2-4 (0) LATITUDE = 39.97608 02.50-04.00 SS-2 10 89 41 48 48 28 20 27 A-7-6 (14) LONGITUDE = -82.87435304.00-05.50 SS-3 89 1.5 BROWN, SILTY CLAY, SOME SAND, TRACE A-6b (VISUAL) GRAVEL 05.50-07.00 SS-4 18 100 3.5 SAME AS SS-3 19 A-6b (VISUAL) B-004-0-19 02.00-03.50 SS-1 10 89 3.75 11 47 34 37 22 15 22 A-6A (10) LATITUDE = 39.976307 BROWN, GRAVEL AND/OR STONE FRAGMENTS WITH SAND, SILT, AND CLAY, FILL A-2-6 (VISUAL) 03.50-05.00 SS-2 16 28 17 LONGITUDE = -82.873364 05.00-06.50 SS-3 25 100 4.5 10 23 A-7-6 (14) GRAY, FINE SAND, TRACE GRAVEL, TRACE SILT 06.50-08.00 SS-4 42 67 3.5 A-3 (VISUAL) B-005-0-19 01.00-02.50 SS-1 57 100 38 19 11 21 11 21 17 4 12 A-2-4 (0) LATITUDE = 39.976451 02.50-04.00 SS-2 37 100 4.5 6 9 13 40 32 33 19 14 15 A-6a (9) LONGITUDE = -82.871843A-6a (VISUAL) 04.00-05.50 SS-3 27 100 4.5 SAME AS SS-2 05.50-07.00 SS-4 28 100 4.5 SAME AS SS-2 14 A-6a (VISUAL) 73 17 B-006-0-19 01.00-02.50 SS-1 100 20 19 11 23 6 8 A-2-4(0)LATITUDE = 39.976579 02.50-04.00 SS-2 39 100 4.5 10 12 17 36 25 24 16 8 11 A-4a (5) LONGITUDE = -82.870332SAME AS SS-2 SS-3 04.00-05.50 25 100 4.5 A-4a (VISUAL) GRAY, SILTY CLAY, SOME SAND, LITTLE GRAVEL, TILL 05.50-07.00 SS-4 A-6b (VISUAL) 24 100 4.5 HAMILTON ROAD (SR 317) B-007-0-19 01.00-02.50 SS-1 9 89 25 23 12 21 19 32 20 12 12 A-6a (2) LATITUDE = 39.973408 02.50-04.00 SS-2 18 100 4.5 0 3 48 48 45 30 15 16 A-7-5 (11) LONGITUDE = -82.87462504.00-05.50 SS-3 16 100 BROWN, SILTY CLAY, TRACE SAND 28 A-6b (VISUAL) 05.50-07.00 SS-4 55 100 4.5 SAME AS SS-3 18 A-6b (VISUAL) B-008-0-19 01.00-02.50 SS-1 21 78 10 41 21 15 9 A-3a (0) LATITUDE = 39.974504 NΡ NΡ 02.50-04.00 SS-2 13 42 22 16 A-1-b (0) LONGITUDE = -82.8742353/4/50/1" 04.00-05.08 SS-3 SAME AS SS-2 A-1-b (VISUAL) B-009-0-19 01.50-03.00 SS-1 10 1.25 47 39 35 21 14 22 67 4 A-6a (10) LATITUDE = 39.975644 53 03.00-04.50 SS-2 9 100 1.25 38 60 27 33 25 A-7-6 (20) LONGITUDE = -82.87389 04.50-06.00 SS-3 100 SAME AS SS-2 A-7-6 (VISUAL) 18 1.5 29 06.00-07.50 SS-4 SAME AS SS-2 A-7-6 (VISUAL) B-010-0-19 01.50-03.00 SS-1 18 100 1.75 42 37 39 24 15 21 A-6a (10) LATITUDE = 39.977115 03.00-04.50 SS-2 12 100 2.5 2 38 51 56 26 30 26 A-7-6 (19) LONGITUDE = -82.873813A-7-6 (VISUAL) 04.50-06.00 SS-3 15 100 4.5 SAME AS SS-2 22 06.00-07.50 SS-4 21 100 2.5 SAME AS SS-2 20 A-7-6 (VISUAL) B-011-0-19 01.50-03.00 19 SS-1 10 100 2.25 6 47 37 37 23 A-6b (12) LATITUDE = 39.978129 03.00-04.50 SS-2 12 100 1.75 53 42 51 26 25 27 A-7-6 (16) LONGITUDE = -82.87387904.50-06.00 13 BROWN, SILTY CLAY, LITTLE SAND, A-6b (VISUAL) SS-3 100 3.25 TRACE GRAVEL 06.00-07.50 A-6b (VISUAL) SS-4 21 100 1.5 SAME AS SS-3 GRAY, GRAVEL AND/OR STONE FRAGMENTS WITH SAND, SILT, AND CLAY, CONTAINS ASPHALT, FILL B-012-0-19 01.50-03.00 SS-1 13 A-2-6 (VISUAL) 28 LATITUDE = 39.9792 LONGITUDE = -82.87355903.00-04.50 SS-2 36 100 4.5 5 10 15 37 33 30 19 13 A-6a (7) 11 27 04.50-06.00 SS-3 100 4.5 SAME AS SS-2 A-6a (VISUAL) 06.00-07.50 43 SAME AS SS-2 A-6a (VISUAL) SS-4 100 4.5

16

(SR .00

TREE

S

⋖

E BRO

Ш

PROFIL

OIL

S

က ထ

ဖ

FRA

50.

368

اري

00

356

ST

 \bigcirc

 \bigcirc

356+00

357+00

358+00

359+00

360+00

361+00

362+00

363+00

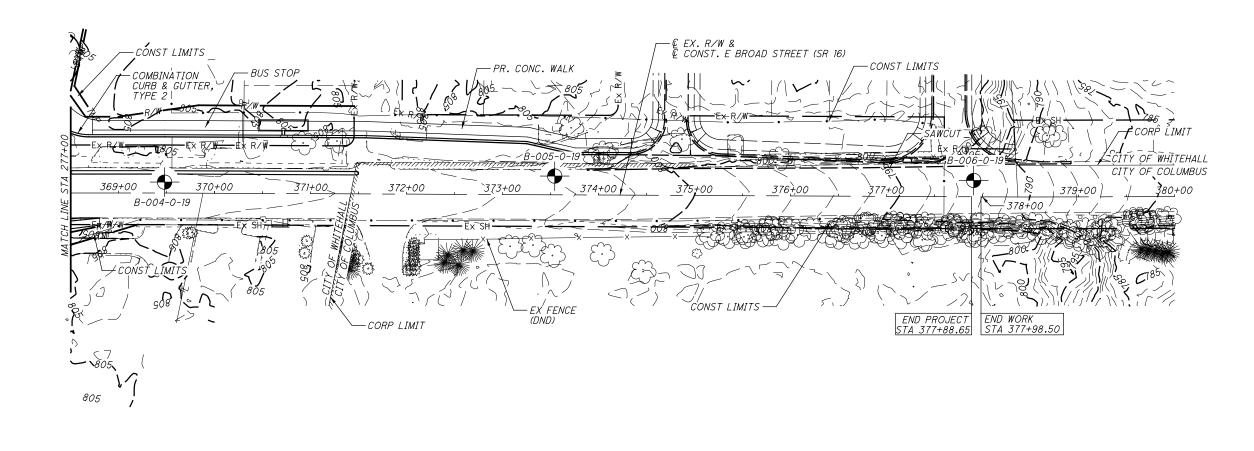
364+00

365+00

366+00

367+00

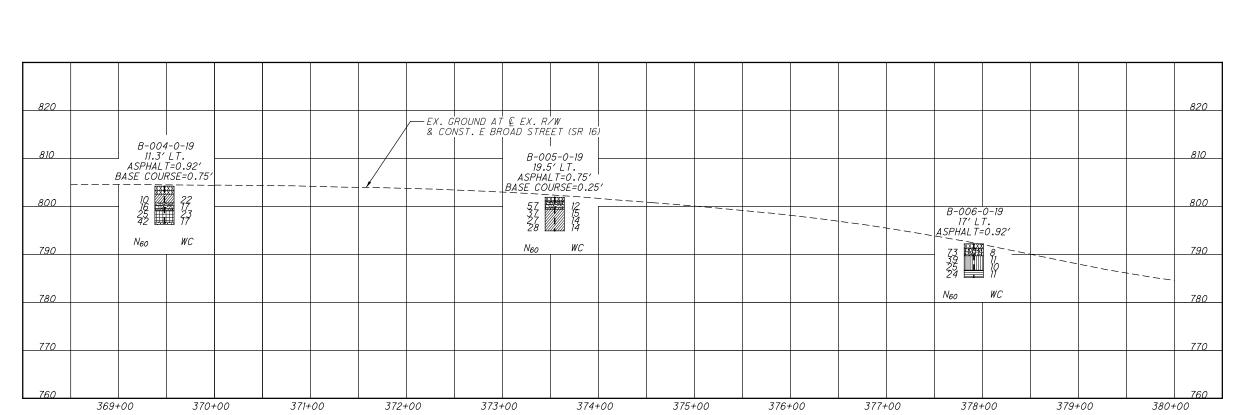
368+00

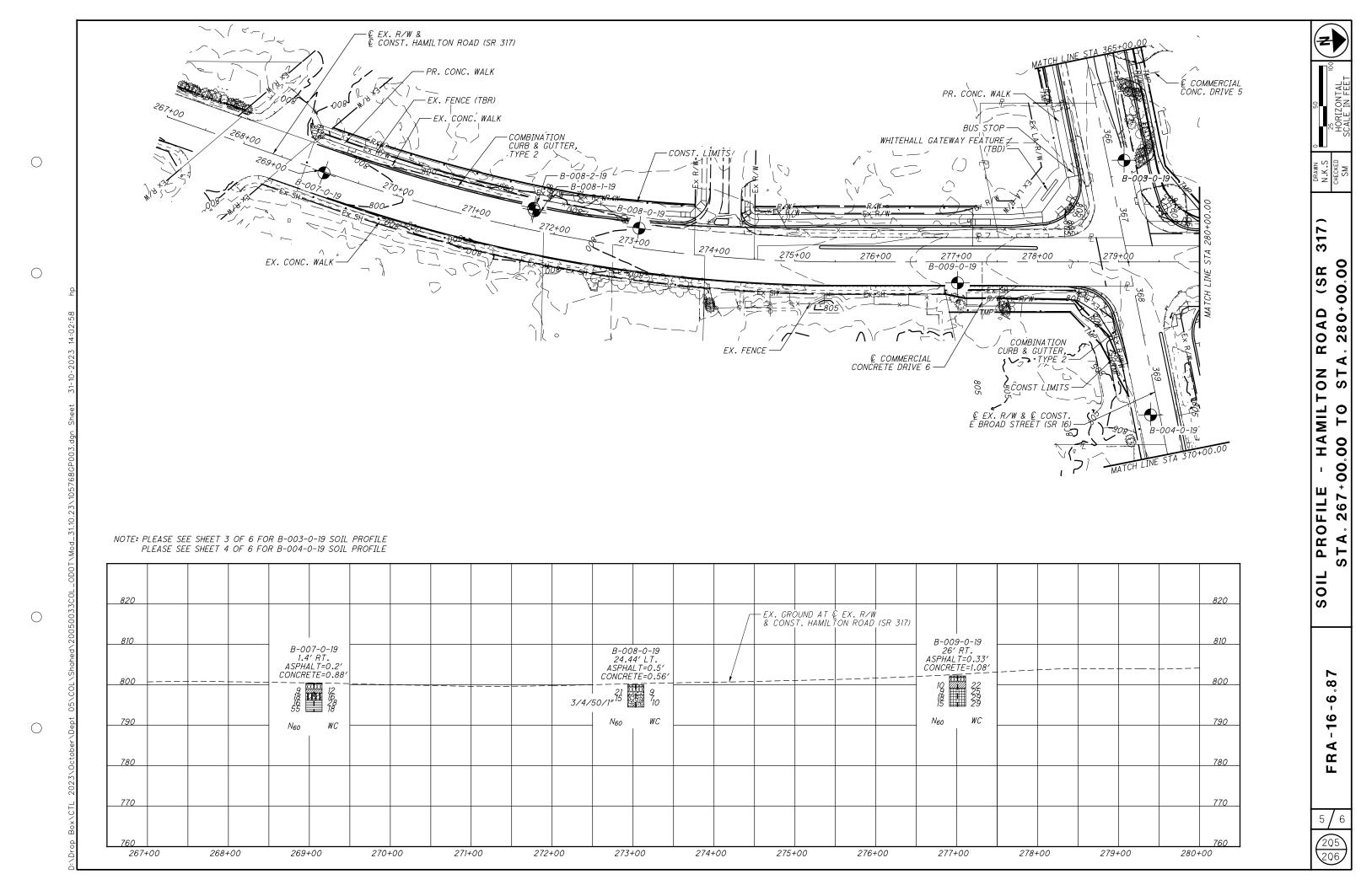


 \bigcirc

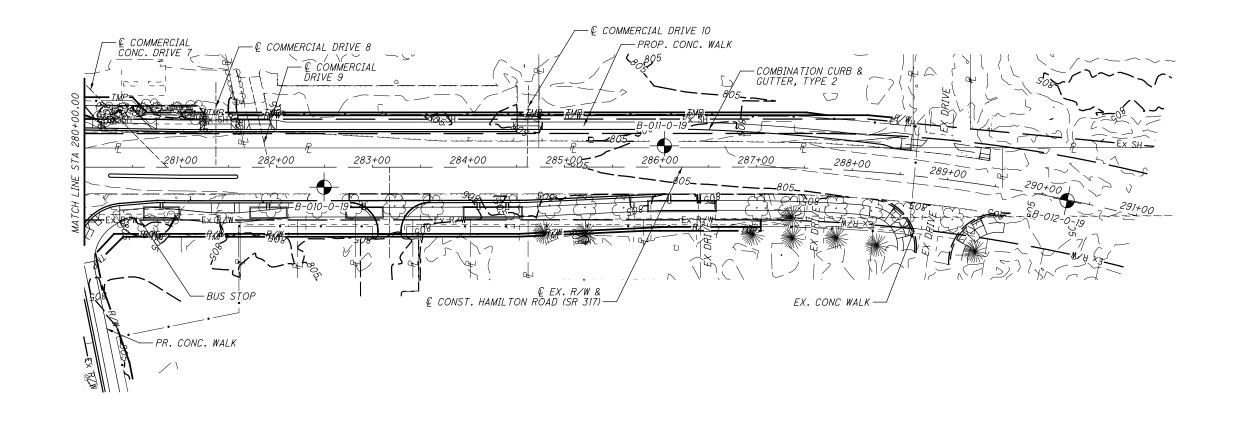
 \bigcirc

 \bigcirc



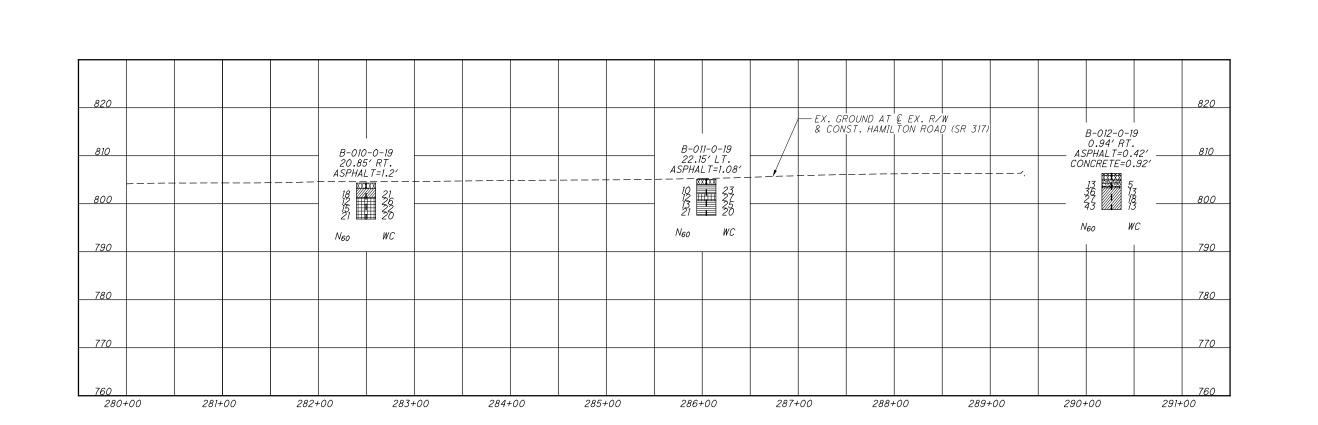


⋖



 \bigcirc

 \bigcirc



APPENDIX B TEST BORING RECORDS



SOIL DESCRIPTION

Descriptors for soil consistency used in this report are based upon the Standard Penetration Test (SPT), ASTM D 1587, with the penetration (N) values corrected to N_{60} , based upon the efficiency of the SPT Hammer used for the soil sampling.

Descriptors for both non-cohesive and cohesive soils are presented below, with the corresponding range of corrected penetration values.

NON-COHESIVE SOIL CORRECTED PENETRATION VALUES **DESCRIPTION BLOWS PER FOOT (BPF)** Very Loose.....0 – 4 Very Dense......Over 50 **COHESIVE SOIL CORRECTED PENETRATION VALUES BLOWS PER FOOT (BPF) DESCRIPTION** Very Soft......0 – 1 Medium Stiff......5 – 8 Hard.....Over 30

SOIL DESCRIPTION	MOISTURE TERMS	DESCRIPTION
Powdery	Dry	Powdery
		Below Plastic Limit
Damp to the Touch	Moist	Above Plastic, Below Liquid Limit
Free Water	Wet	Above Liquid Limit
		•

COHESIVE SOIL

Moisture term descriptors for both non-cohesive and cohesive soils are presented below.

NON-COHESIVE



PROJECT: FRA-16-6.87 DRILLING FIRM / OPER		DRILL RIG: CME 45-TRACK 509 STATION / OFFSET: 358+80, 7' RT. EXPLO B-0						
TYPE: ROADWAY SAMPLING FIRM / LOGO PID: 105768 SFN: DRILLING METHOD: START: 8/19/20 END: 8/19/20 SAMPLING METHOD:	GER: <u>CTL / DONOVAN</u> 3.25" HSA SPT	5" HSA CALIBRATION DATE: 8/5/20 ELEVATION: 800.5 (MSL) EOB:						
MATERIAL DESCRIPTION AND NOTES	ELEV. DEPTHS 800.5	SPT/ RQD N ₆₀ REC (%)	SAMPLE HP ID (tsf) GF	GRADATION (%)	ATTERBERG LL PL PI WO	ODOT BACK FILL		
Asphalt (15")	799.3							
Base course (3") VERY STIFF, BROWN, SANDY SILT , SOME CLAY, TRACE GRAVEL, DAMP	799.0	13 8 10 27 100	SS-1 4.00 7	13 17 37 26	31 21 10 18	A-4a (6)		
VERY STIFF, BROWN, CLAY, "AND" SILT, LITTLE SAND, TRACE GRAVEL, DAMP		3 3 10 89	SS-2 2.75 1	2 9 36 52	54 25 29 23	A-7-6 (18) $\begin{array}{c} 7 & 7 & 7 \\ 4 & 7 & 7 \\ 7 & 7 & 7 \end{array}$		
	- 5 -	⁴ 5 15 100	SS-3 2.75 -		21	A-7-6 (V)		
@6.0'; MEDIUM STIFF, BROWN AND GRAY, SOME SAND, LITTLE SILT, LITTLE GRAVEL, MOIST	793.0 FOR 7	5 4 3 10 56	SS-4 0.50 -		32	< . \ < .		

PROJECT	PROJECT: FRA-16-6.87 DRILLING FIRM / OPERATOR: CTL / DO						DRIL	L RIG	CN	1E 45-TRA	ACK 50	9_	STAT	ION	OFF:	SET:	36	3+19	, 10'	LT.	EXPLOR	
TYPE:	ROA	DWAY	SAMPLING FIRM / LOG	GER:	CTL / DONO	VAN	HAMMER: <u>CME AUTOMATIC</u> ALIGNMENT: B-C								B-002	2-0-19						
PID:10	5768 SFN:		DRILLING METHOD: _	3	3.25" HSA		CALI	BRAT	ON DA	ATE:	8/5/20		ELEVATION: 802.2 (MSL) EOB: 7.0 ft.							.0 ft.	PAGE	
START: _	8/20/20 EI	ND: <u>8/20/20</u>	20/20 SAMPLING METHOD: SPT						OITAS	(%):	89.3		LAT /	LON	G: _		39.97	6075	, -82.	.87560)1	1 OF 1
	M	ATERIAL DESCRIPT	ELEV.	DEPTH	IC.	SPT/	N ₆₀	REC	SAMPLE	HP		GRAD	ATIC	N (%))	ATTE	ERBE	RG		ODOT	BACK	
ž.		802.2	DEFIN	13	RQD	1N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL		
Asphalt (1	12")	801.2		 - 1 -																		
1	, ,	SILT AND CLAY, SO NS COBBLES, DAM	·			- ' - 2 -	15 7 8	22	67	SS-1	-	28	9	9	30	24	32	20	12	18	A-6a (5)	12 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
@2.5'; HA	ARD, TRACE G	RAVEL		798.2		- 3 -	5 6 8	21	100	SS-2	4.50	3	8	12	38	39	38	23	15	19	A-6a (10)	12 12 12 12 12 12 12 12 12 12 12 12 12 1
HARD, BI GRAVEL,		CLAY, SOME SAND), LITTLE			- 4 - 5 -	7 7 10	25	100	SS-3	4.50	-	-	1	-	-	-	-	-	13	A-6b (V)	1> \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
@5.5'; VE	@5.5'; VERY STIFF					- 6 -	7 5 5	15	100	SS-4	3.00	-	-	-	-	-	-	-	-	19	A-6b (V)	1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 ×

PR	OJECT: _		FRA-16	5-6.87	DRILLING FIRM / O	PERA	RATOR: CTL / DONOVAN			DRILL RIG: <u>CME 45-TRACK 509</u>						STAT	TION .	OFF	SET:	: <u>366+33, 20' RT.</u>				EXPLOR	-
TY	PE:	R	OADWA	AY	SAMPLING FIRM / L	OGG	ER:	CTL / DONG	NAVC	HAM	MER:	CN	ME AUTO	MATIC		ALIG	NME	NT: _						B-003	3-0-19
PIE	1057	68 SF	N:		DRILLING METHOD):	3.25" HSA CALIBRATION DATE: 8/5/20 ELE\				ELEVATION: 803.1 (MSL) EOB: 7.0 ft.						.0 ft.	PAGE							
ST	START: 8/19/20 END: 8/19/20 SAMPLING METHOD: SPT						ENE	RGY F	ATIO ((%):	89.3		LAT /	LON	IG: _		39.9	76080), -82	.87435	53	1 OF 1			
3	MATERIAL DESCRIPTION						ELEV.	DEPT	пe	SPT/	N ₆₀	REC	SAMPLE	HP		GRAD	ATIC	N (%)	ATT	ERBE	ERG		ODOT	BACK
3	AND NOTES						803.1	DEFI	по	RQD	1N ₆₀	(%)	ID	(tsf)	GR	cs	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL
As As	phalt (10"	')				\bowtie	802.3																		
יוי, ∕	se course	` '					802.0		<u></u> 1	15	00		00.4		0.5	1	40	0.4			40		40	4.0.4.(0)	*********
,		,	- ,	GRAVEL AND/O AND SILT, LITTL		NΩ	800.6		_ 2 -	11 9	30	56	SS-1	-	35	21	12	21	11	22	18	4	12	A-2-4 (0)	12/12
	ONTAINS				L OLAT, /				F 3 +	3	10	89	SS-2	2.00	1	2	a	41	48	48	28	20	27	A-7-6 (14)	< \/ <
			AY, "AN	D" SILT, LITTLE	SAND, TRACE		<u> 799.1</u>			4		00	00-2	2.00	_ '			71	70	70	20	20	21	7-7-0 (14)	1>11>
?	RAVEL, D		=:		- <u>-</u>				├	2	7	89	SS-3	1.50		_	_	_	_	١.		_	28	A-6b (V)	7/1/1/
			TY CLA	Y , SOME SAND	, TRACE				5 1	3			00-0	1.50										/(OB (V)	1 L V 1 L
	RAVEL, D. 5.5'; VER`								6	4 5	18	100	SS-4	3.50	_	_	_	_	_	_	_	_	20	A-6b (V)	1>11>
							796.1	L_FOR_	<u></u>	7													'	1 (1)	1 LV 1 L

	PROJECT:	FRA-16-6.87 ROADWAY	DRILLING FIRM / OPERA SAMPLING FIRM / LOGG	_	CTL / DONOVAN				ME 45-TRA			STAT ALIG				3	69+48	3, 11'	LT.	EXPLORA B-004	
	PID: <u>105768</u>		DRILLING METHOD:		.25" HSA		.IBRAT			3/5/20		ELEV				(MS	L)_ E	OB:	8	3.0 ft.	PAGE
ار ا	START: <u>8/18</u>	/20 END: 8/18/20	SAMPLING METHOD:		SPT	ENE	RGY F	_		89.3	_	LAT /							.8733	64	1 OF 1
50033C		MATERIAL DESCRIPT	TON	ELEV.	DEPTHS	SPT/			SAMPLE			GRAD			_		ERBE			ODOT CLASS (GI)	BACK
20050	Asphalt (11")	AND NOTES	XX	804.2		RQD	- 00	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL
068/2	Base course (9	')		803.3 802.6	- 1	-															
EPORTS/L		BROWN, Silt and Clay , Li ^e El, Fill, Damp		800.7	_ 2 _ 3	6 4	10	89	SS-1	3.75	2	6	11	47	34	37	22	15	22	A-6a (10)	<pre></pre> <pre><</pre>
3-6.87\R		SE, BROWN, GRAVEL AND/O WITH SAND, SILT, AND CLA	OR STONE Y, FILL, WET	799.2	- 4	3 4	16	28	SS-2	-	-	-	-	-	-	-	-	-	17	A-2-6 (V)	1 > L 1 > L
-FRA-16	HARD, BROWI GRAVEL, MOIS	N, CLAY , "AND" SILT, LITTLE ST	OR STONE Y, FILL, WET SAND, TRACE		- 5 - 6	5 7	25	100	SS-3	4.50	3	4	10	41	42	44	21	23	23	A-7-6 (14)	1> N 1 Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y
EMH-T	@6.5'; VERY S	TIFF, DAMP , FINE SAND , TRACE, GRAV	FL TRACE SILT (1989)	796.7 796.2	7	6	42	67	SS-4	3.50	-	-	-	-	-	-	-	-	17	A-3 (V)	<pre></pre> <pre><</pre>
033CO	DAMP	, I INC CAND, TIVACE, CIVAV	LE, HVIOL GIET,		EOB8-		-				•				•					•	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
120050																					
SJECTS																					
1/20 PRC																					
NDEPTE																					
9:23 - J:																					
15/20 0																					
DT - 10/																					
DOT.G																					
11) - OH																					
(8.5 X																					
16 L06																					
- BORIN																					
STANDARD ODOT SOIL BORING LOG (8.5 X 11) - OH DOT.GDT - 10/15/20 09:23 - J.\DEPT5/20 PROJECTS\20050033CO																					
RD OD(
TANDA																					
S	NOTES NON																				

PROJECT:	FRA-16-6.8	7	DRILLING FIRM / OPER	RATOR: _	CTL / DONOVAN	DRIL	L RIG	: <u>CN</u>	/IE 45-TRA	CK 509	9_	STAT	TION .	OFF	SET:	3	73+54	1, 20'	LT.	EXPLOR/	
TYPE:	ROADWAY		SAMPLING FIRM / LOG	GER:	CTL / DONOVAN	_ HAM	MER:	CI	ME AUTON	MATIC		ALIG	NME	NT: _						B-005	
PID: 105768	SFN:		DRILLING METHOD:	3	.25" HSA	CALI	BRAT	ION D	ATE:8	3/5/20		ELEV	/ATIC	N: _8	301.9	(MSI	L)_E	OB:	7	.0 ft.	PAGE
	20 END:8	8/18/20	SAMPLING METHOD:		SPT	ENE	RGY F	OITAS	(%):	89.3		LAT /	LON	G:		39.9	76451	, - 82	.87184	3	1 OF 1
	MATERIAL	DESCRIPT	ON	ELEV.	DEDTHS	SPT/	N	REC	SAMPLE	HP		GRAD	ATIC	N (%))	ATT	ERBE	RG		ODOT	BACK
	AND	NOTES		801.9	DEFINS	RQD	11160	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	wc	CLASS (GI)	FILL
Asphalt (9")			\boxtimes	801.1]																
∖Base Course (3	")			<u> </u>	<u> </u>	12															<pre></pre> <pre><</pre>
				799.4	_ 2 -	16 22	57	100	SS-1	-	38	19	11	21	11	21	17	4	12	A-2-4 (0)	7>/ 7>
DAMP					3 -	6 8 17	37	100	SS-2	4.50	6	9	13	40	32	33	19	14	15	A-6a (9)	1 2 1 2 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2
					- 4 - - 5 -	6 8 10	27	100	SS-3	4.50	-	-	-	-	-	-	-	-	14	A-6a (V)	1> \ 1 > \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
				794.9	EOB 7	6 7 12	28	100	SS-4	4.50	-	-	-	-	-	-	-	-	14	A-6a (V)	1 > V 1 > V
	TYPE:	TYPE: ROADWAY PID: 105768 SFN: START: 8/18/20 END: 8 MATERIAL AND Asphalt (9") Base Course (3") VERY DENSE, BROWN, GRAVE FRAGMENTS WITH SAND AND DAMP	TYPE: ROADWAY PID: 105768 SFN: START: 8/18/20 END: 8/18/20 MATERIAL DESCRIPTION AND NOTES Asphalt (9") Base Course (3") VERY DENSE, BROWN, GRAVEL AND/ORS FRAGMENTS WITH SAND AND SILT, LITTL DAMP HARD, BROWN AND GRAY, SILT AND CLATRACE GRAVEL, DAMP	TYPE: ROADWAY PID: 105768 SFN: DRILLING METHOD: START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SAMPLING METHOD: MATERIAL DESCRIPTION AND NOTES Asphalt (9") Base Course (3") VERY DENSE, BROWN, GRAVEL AND/OR STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, FILL, DAMP HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP	TYPE: ROADWAY PID: 105768 SFN: DRILLING METHOD: 3 START: 8/18/20 END: 8/18/20 SAMPLING METHOD: 3 MATERIAL DESCRIPTION AND NOTES Asphalt (9") Base Course (3") VERY DENSE, BROWN, GRAVEL AND/OR STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, FILL, DAMP HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP (94.0'; BROWN	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN PID: 105768 SFN: DRILLING METHOD: 3.25" HSA START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT MATERIAL DESCRIPTION ELEV. B01.9 AND NOTES 801.9 Base Course (3") 800.9 1 VERY DENSE, BROWN, GRAVEL AND/OR STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, FILL, DAMP 1 HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 4 @4.0'; BROWN GRAYEL AND/OR STONE 1 Contact 1 Conta	TYPE:	TYPE:	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN HAMMER: CI	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN HAMMER: CME AUTON CALIBRATION DATE: START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): SPT RQD N60 REC SAMPLE RQD N60 RQD RQD	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN HAMMER: CME AUTOMATIC START: 8/18/20 END: 8/18/20 SAMPLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 SAMPLING METHOD: SPT SAMPLE SAMPLE	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DRILLING METHOD: 3.25" HSA START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 MATERIAL DESCRIPTION AND NOTES SPT SPT SPT RQD N ₆₀ REC SAMPLE HP (%) ID (tsf) GR	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN HAMMER: CME AUTOMATIC CALIBRATION DATE: 8/5/20 ELEV ENERGY RATIO (%): 89.3 LAT / ENERGY RATIO (%): 89.3 LAT / ENERGY RATIO (%): 89.3 LAT / ENERGY RATIO (%): 80.1 LAT / ENERGY RATIO (%): 80.3 LAT / ENERGY RATIO (%): 80.3 LAT / ENERGY RATIO (%): 80.1 LAT / ENERGY RATIO (%): 80.1 LAT / ENERGY RATIO (%): 80.3 LAT / ENERGY RATIO (%): 80.1 LAT / ENERGY RATIO (%): 80.1 LAT / ENERGY RATIO (%): 80.3 LAT / ENERGY R	TYPE: ROADWAY PID: 105768 SFN: DRILLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ELEVATIO (%): 89.3 LAT / LON	TYPE: ROADWAY PID: 105768 SFN: DRILLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ELEVATION: ELEVATION: END: SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 LAT / LONG: LAT / LONG:	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DRILLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ELEVATION: 801.9 START: 8/18/20 END: 8/18/20 END: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 LAT / LONG: ELEVATION: 801.9 SPT ELEVATION: 801.9 SPT ELEVATION: 801.9 SPT SPT	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DID 105768 SFN: DRILLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ELEVATION DATE: 8/5/20 ENERGY RATIO (%): 89.3 LAT / LONG: 39.9	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DRILLING METHOD: 3.25" HSA SAMPLING METHOD: SPT START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 LAT / LONG: 39.976451 SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 LAT / LONG: 39.976451 SAMPLING METHOD: SPT SPT ROD N ₆₀ (%) ID (tsf) GR CS FS SI CL LL PL RAGMENTS WITH SAND AND SILT, LITTLE CLAY, FILL, DAMP HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP (@4.0"; BROWN) BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP (@4.0"; BROWN) SROWN SS-4 4.50 C C C C C C C C C	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN HAMMER: CME AUTOMATIC CALIBRATION DATE: 8/5/20 ELEVATION: 801.9 (MSL) EOB: 239.976451, -82	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DRILLING METHOD: 3.25" HSA SAMPLING METHOD: 3.25" HSA SAMPLING METHOD: SPT ELEVATION: 801.9 MSL) EOB: 7. EDEPTHS SPT RQD No. REC SAMPLE HP GRADATION (%) BASE Course (3") SAMPLING REVEL AND/OR STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, FILL, DAMP HARD, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP (@4.0"; BROWN BROWN BROWN BROWN REC SAMPLE HP GRADATION (%) ATTERBERG SAMPLE HP SAMPLE HP	TYPE: ROADWAY SAMPLING FIRM / LOGGER: CTL / DONOVAN DILLING METHOD: 3.25" HSA START: 8/18/20 END: 8/18/20 SAMPLING METHOD: SPT ENERGY RATIO (%): 89.3 LAT / LONG: 39.976451, -82.871843 MATERIAL DESCRIPTION AND NOTES SUBJECT OF START SOME SAMPLING METHOD: SPT SPT SPT RQD N ₆₀ REC SAMPLE HP GRADATION (%) ATTERBERG RQD RQD

- 1	PROJECT: FRA-16-6.87 TYPE: ROADWAY	DRILLING FIRM / OPERA SAMPLING FIRM / LOGG		CTL / DONO		-	L RIG MER:		ME 45-TRA			STAT ALIG		OFF	SET:	37	77+91	1, 17'	LT.	EXPLOR B-006	ATION ID S-0-19
2	PID: 105768 SFN:	DRILLING METHOD:	3.	25" HSA		CALI	BRAT	ON DA		3/5/20		ELEV	/ATIC	N: _7	792.2	(MSL	_)_ E	OB:	7	.0 ft.	PAGE
jL	START: <u>8/18/20</u> END: <u>8/18/20</u>	SAMPLING METHOD:		SPT		_ ENEF	RGY F	PATIO ((%):	89.3		LAT /	LON	G:		39.97	76579	, -82	.87033	32	1 OF 1
55 5	MATERIAL DESCRIP	TION	ELEV.	DEPTH	16	SPT/	NI	REC	SAMPLE	HP	0	GRAD	ATIC	N (%))	ATT	ERBE	RG		ODOT	BACK
35	AND NOTES		792.2	DEFIR	13	RQD	N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	L	PL	ΡI	WC	CLASS (GI)	FILL
SIZU	Asphalt (11")		791.3																		
S/LOG	VERY DENSE, BROWN, GRAVEL AND/OR FRAGMENTS WITH SAND AND SILT, LITT DAMP		789.7		- 1 - - 2 -	12 22 27	73	100	SS-1	-	41	20	9	19	11	23	17	6	8	A-2-4 (0)	7 L V
וארוי סייר	HARD, BROWN, SANDY SILT , SOME CLA' GRAVEL, DAMP	7, LITTLE			- 3 - - - 4 -	11 12 14	39	100	SS-2	4.50	10	12	17	36	25	24	16	8	11	A-4a (5)	1 > \ 1 > \
4-10-0.0			786.7		- - 5 -	10 8 9	25	100	SS-3	4.50	-	-	-	-	-	-	-	-	10	A-4a (V)	1>\ 1>\ 1>\ 1>
H-I-R	HARD, GRAY, SILTY CLAY , SOME SAND, TILL, DAMP	LITTLE GRAVEL,	785.2	FOR—	- 6 -	5 8 8	24	100	SS-4	4.50	-	-	-	-	-	-	-	-	11	A-6b (V)	1>V 1>

- 1	PROJECT: FRA-16-6.87 TYPE: ROADWAY	DRILLING FIRM / OPERA SAMPLING FIRM / LOGG		CTL / DONC		DRIL	L RIG:		IE 45-TRA			STAT ALIG		/ OFF NT:	SET:	2	69+0	6, 1' F	RT.	EXPLOR B-007	ATION ID 7-0-19
ויִי	PID: 105768 SFN:	DRILLING METHOD:	3.	25" HSA		CALII	BRATI	ON DA	TE:8	3/5/20				_		(MSL				.0 ft.	PAGE 1 OF 1
₹.	START: <u>8/19/20</u> END: <u>8/19/20</u>	SAMPLING METHOD:		SPT		ENE	RGY F	ATIO (%):	89.3		LAT /	LON	IG: _		39.97	⁷ 3408	8, -82	.87462	25	TOFT
35	MATERIAL DESCRIPT	TION	ELEV.	DEPTI	J0	SPT/	N ₆₀	REC	SAMPLE	HP	(SRAD	ATIC	N (%)	ATT	ERBE	RG		ODOT	BACK
젌	AND NOTES		800.4	DEFII	13	RQD	1160	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	FILL
SIZUL	Asphalt (2")		800.2		- , -																
I S/LOG	Concrete (10.5") STIFF, BROWN, SILT AND CLAY, SOME S GRAVEL, DAMP	AND, SOME	798.9 797.9		- 1 - - 2 -	3 3 3	9	89	SS-1	-	25	23	12	21	19	32	20	12	12	A-6a (2)	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
ארבוי היי	HARD, BROWN, ELASTIC CLAY , "AND" SII DAMP	LT, TRACE SAND,	796.4		3 -	5 5 7	18	100	SS-2	4.50	0	1	3	48	48	45	30	15	16	A-7-5 (11)	7 L 7 L 7 > C 7 L
4-10-0.0	VERY STIFF, BROWN, SILTY CLAY , TRAC GRAVEL, MOIST	E SAND, TRACE			- 4 - 5 -	4 5 6	16	100	SS-3	-	-	-	-	-	-	-	-	-	28	A-6b (V)	1>\ 1>\ 1>
-I-FR	@5.5'; HARD, GRAY, CONTAINS SHALE FF	RAGMENTS, DAMP	793.4	EOR—	- 6 - - 7-	6 7 30	55	100	SS-4	4.50	-	-	-	-	-	-	-	-	18	A-6b (V)	1>V 1>

PROJE	CT: FRA-	16-6.87	DRILLING FIRM / OPER/	ATOR:	CTL / DONOVAN	DRIL	L RIG	_ CM	IE 45-TRA	CK 509	9_	STAT	ION /	OFF	SET:	2	73+03	3, 24'	LT.	EXPLORA	-
TYPE:	ROAD\	VAY	SAMPLING FIRM / LOGO	SER: C	TL / DONOVAN	HAM	IMER:	CN	ME AUTON	MATIC		ALIGN	MEN	NT: _						B-008	3-0-19
PID:	105768 SFN:		DRILLING METHOD:	3.	25" HSA	CALI	BRATI	ON DA	ATE: 8	3/5/20		ELEV	ATIO	N: 8	300.0	(MS	L) E	OB:	5	.5 ft.	PAGE
START	8/19/20 END	: 8/19/20	SAMPLING METHOD: _		SPT	ENE	RGY F	ATIO ((%):	89.3		LAT/	LON	G:		39.9	74504	1, - 82	.87423	5	1 OF 1
	MAT	ERIAL DESCRIPT	TON	ELEV.	DEPTHS	SPT/	NI.	REC	SAMPLE	HP	(GRAD.	ATIO	N (%)	ATT	ERBE	RG		ODOT	BACK
		AND NOTES		800.0	DEPTINO	RQD	N ₆₀	(%)	ID	(tsf)	GR	cs	FS	SI	CL	Ц	PL	PI	WC	CLASS (GI)	FILL
Aspha	t (6")			799.5															i		
Concr	ete (6.75")		\bowtie	798.3		7													i		<pre>// / / / / / / / / / / / / / / / / / /</pre>
MEDIL	JM DENSE. BROWN	COARSE AND	FINE SAND,	797.5	- 2 -	6	21	78	SS-1	-	10	41	21	17	11	19	15	4	9	A-3a (0)	1>11>
	E CLAY, TRACE SIL	,	L, FILL, DAMP			8															1 LV 1 L
	JM DENSE, BROWN			d		5 _	15	89	SS-2	-	13	42	22	16	7	NP	NP	NP	7	A-1-b (0)	1>11>
n -	MENTS WITH SAND), LITTLE SILT, TF	RACE CLAY, FILL,	3	⊢ 4 −	3															12/12
DAMP			à 🔾	3		4	-	100	SS-3	-	-	-	-	-	-	-	-	-	10	A-1-b (V)	1 × 1 × 1
<u> </u>				794.5	EOB J	50/1"/													لــــــــــــــــــــــــــــــــــــــ		1/2//2

NOTES: SPOON REFUSAL ON CONCRETE AT 5.5 FEET

PROJECT: FRA-16-6.87 DRILLING FIRM / OF	ERATOR: C	TL / DONOVAN	DRILL RIG	i: <u>CN</u>	IE 45-TRAC	CK 509	_ STA	TION	OFFS	SET:	277+	01, 26	RT.		ATION ID
TYPE: SAMPLING FIRM / LO	GGER: <u>C1</u>	TL / DONOVAN	HAMMER:	CI	ME AUTOM	ATIC	_ ALIG	SNME	NT:					B-008	9-0-19
PID: <u>105768</u> SFN: DRILLING METHOD:	3.2	25" HSA	CALIBRAT	ION D	ATE:8/	5/20	_ ELE	VATIC	N: <u>8</u>	302.2	(MSL)	EOB:	7	.5 ft.	PAGE
START: <u>8/20/20</u> END: <u>8/20/20</u> SAMPLING METHOD	:	SPT	ENERGY I	RATIO	(%):8	9.3	_ LAT	/ LON	IG:		39.9756	44, -82	2.87389	90	1 OF 1
MATERIAL DESCRIPTION	ELEV.	DEPTHS	SPT/ N ₆₀	REC	SAMPLE	HP	GRAI	DATIC	N (%)		ATTER	BERG		ODOT	BACK
AND NOTES	802.2	DEFINS	RQD No.	(%)	ID	(tsf)	R CS	FS	SI	CL	LL PI	_ PI	wc	CLASS (GI)	FILL
Asphalt (4")	801.9														
Concrete (13")	8.008	_ 1 _													
STIFF, BROWN AND GRAY, SILT AND CLAY , LITTLE SAND, TRACE GRAVEL, MOIST	799.2	_ 2 _	3 3 10	67	SS-1	1.25	1 4	9	47	39	35 2·	1 14	22	A-6a (10)	47.47
STIFF, BROWN, CLAY , "AND" SILT, TRACE SAND, TRACE GRAVEL, DAMP		- 3 - - 4 -	2 3 9	100	SS-2	1.25	1 2	6	38	53	60 2	7 33	25	A-7-6 (20)	1 × × × × × × × × × × × × × × × × × × ×
@4.5'; BROWN AND GRAY, LITTLE SAND, MOIST		5 -	3 4 18	100	SS-3	1.50	- -	-	-	-	- -	-	29	A-7-6 (V)	1>11>
@6.0'; LITTLE GRAVEL	794.7	- 0 T	3 4 15 6	100	SS-4	1.00	- -	-	-	-		-	29	A-7-6 (V)	< \/ < .

PROJI		FRA-16-6.87 ROADWAY	DRILLING FIRM / OPER	_	CTL / DON		- 1	L RIG	-	1E 45-TRA			STAT		/ OFF NT:	SET:	2	82+50), 21'	RT.	_	ATION ID 0-0-19
PID:		SFN:	DRILLING METHOD:	3	.25" HSA		- -		ON DA		8/5/20		ELE\		_			L)_ E			.5 ft.	PAGE 1 OF 1
STAR	Γ:8/20/20	END:8/20/20 MATERIAL DESC	SAMPLING METHOD: _	ELEV.	SPT T		_ ENEI SPT/		ATIO	(%):	89.3 HP		LAT /	_	IG: DN (%	_		ERBE	_	.8738 ⁻		BACK
		AND NOTE		804.3	DEPT	HS	RQD	N ₆₀	(%)	ID	(tsf)	GR	_	FS	SI	CL	LL	PL	PI	wc	ODOT CLASS (GI)	
<u> </u>	alt (14")			803.2		- 1 -																
	F, BROWN A CE GRAVEL,	ND GRAY, SILT AND DAMP	CLAY, LITTLE SAND,	801.3		_ 2 -	4 5 7	18	100	SS-1	1.75	8	4	9	42	37	39	24	15	21	A-6a (10)	4 / 4 /
		DWN AND GRAY, CLA ACE GRAVEL, DAMP	Y, "AND" SILT,			- 3 - - 4 -	3 4 4	12	100	SS-2	2.50	2	2	7	38	51	56	26	30	26	A-7-6 (19)	1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2
@4.5	'; HARD, BR	NWC				_ 5 -	3 4 6	15	100	SS-3	4.50	-	-	-	-	1	-	-	1	22	A-7-6 (V)	1 > L 1 > L
@6.0	'; VERY STIF	F		796.8	FOR—	- 6 - - 7 -	4 6 8	21	100	SS-4	2.50	-	-	-	-	-	-	-	-	20	A-7-6 (V)	1 > L 1 > L

PROJE	ECT:	FRA-16-		DRILLING FIRM / OF SAMPLING FIRM / L			CTL / DON		-	L RIG		1E 45-TRA			STA1 ALIG			SET:	2	86+04	4, 22'	LT.	EXPLOR B-011	ATION ID 1-0-19
PID: _		SFN:		DRILLING METHOD	:		25" HSA SPT		CALI	BRATI	ON DA	ATE:	89.3	_	ELE\	/ATIC)N: _			L)_ E		.87387	7.5 ft.	PAGE 1 OF 1
S S S S S S S S S S S S S S S S S S S	. <u> </u>	MATER	RIAL DESCRIPT AND NOTES		<u> </u>	ELEV. 805.3	DEPT	'HS	SPT/ RQD	N ₆₀	_	SAMPLE ID	_	=	GRAD				_	ERBE	_	wc	ODOT CLASS (GI)	BACK FILL
<u> </u>	alt (13")	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	V CLAV LITTLE	SAND, TRACE		804.2		- - 1 -																- 5 LV 5 L
J	EL, MOIST	JVVIN, SILI	T CLAT, LITTLE	E SAND, TRACE		802.3		_ 2 -	3 3 4	10	100	SS-1	2.25	1	6	9	47	37	37	18	19	23	A-6b (12)	Lisk is
STIFF	, BROWN, C	CLAY, "ANI	D" SILT, TRACE	SAND, MOIST		800.8		- 4 -	3 4 4	12	100	SS-2	1.75	0	1	4	53	42	51	26	25	27	A-7-6 (16)	1>V1> 1>V1> 1>V1>
	STIFF, BRO EL, MOIST	OWN, SILT	Y CLAY, LITTLE	SAND, TRACE				5 -	3 4 5	13	100	SS-3	3.25	-	-	-	-	-	-	-	1	25	A-6b (V)	1>N 1> 1 L 1 7 L
@6.0	; STIFF					797.8	EOB-	- 7 -	5 6 8	21	100	SS-4	1.50	-	-	-	-	-	-	-	-	20	A-6b (V)	7 L V 7 L V X V X V X V X V X V X V X V X V X V

- 1	PROJEC [®]	:T:		A-16-6		DRILLING FIRM			CTL / DOI			L RIG		1E 45-TRA			STAT			SET:	2	90+2	.6, 1' I	RT.	EXPLOR/ B-012	
	TYPE: _ PID:1	05768	ROAI	JWAY	<u>/</u>	SAMPLING FIRM DRILLING METH			CTL / DON 25" HSA	OVAN	•	MER: BRATI	ON DA	ME AUTON ATE:8	3/5/20		ALIG ELE\		_	306.3	(MS	L)_ E	OB:	7	.5 ft.	PAGE
Ö	START:	8/20/2	<u>0 EN</u>	ID:	8/20/20	SAMPLING MET	HOD:		SPT		ENE	RGY F	ATIO ((%):	89.3		LAT /	LON	IG: _		39.9	79200	0, -82	.87355	59	1 OF 1
330			MA	TERI	AL DESCRIPT	TON		ELEV.	DEP.	TLIC	SPT/	N	REC	SAMPLE	HP		GRAD	ATIC	N (%)	ATT	ERBE	ERG		ODOT	BACK
2000				Α	ND NOTES			806.3	DEF	1110	RQD	N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL
)GS/20(Asphalt Concrete	` '					-/ 💥	805.9 805.0		- 1 -																
ORTS/LC		IENTS W	ÍTH SAN		AVEL AND/OR ILT, AND CLA	STONE Y, CONTAINS		803.3		_ 2 _	17 4 5	13	28	SS-1	-	-	-	-	-	-	-	-	-	5	A-2-6 (V)	<pre></pre> <pre><</pre>
3.87\REP	<u> </u>	BROWN,		ND CI	LAY, SOME S	AND, TRACE	- / ///			4	7 10 14	36	100	SS-2	4.50	5	10	15	37	33	30	19	11	13	A-6a (7)	1> \ 1 \ \ 1
:RA-16-										5 -	5 6 12	27	100	SS-3	4.50	-	-	-	-	-	-	-	-	18	A-6a (V)	1> \ 1 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
EMH-T-F								798.8	EOB-	- 6 - - 7 -	11 13 16	43	100	SS-4	4.50	-	-	-	-	-	-	-	-	13	A-6a (V)	1>N 1>

APPENDIX C PAVEMENT CORE REPORT



PAVEMENT CORE REPORT

Broad Street and Hamilton Road Intersection Improvements FRA-16-6.87, PID No. 105768 CTL Project No.: 20050033COL

Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-001-0-19	08/19/2020	6.0"	719860.6130	1862826.1010	800.543



Depth (inches)	Pavement Type	Notes
0 - 6.0	Asphalt Concrete	Surface Course
6.0 - 15.0	Asphalt Concrete	Intermediate Course

Recovered Core Length	In-hole Depth	Recovery
(inches)	(inches)	(%)
15.0	15.0	100



PAVEMENT CORE REPORT

Broad Street and Hamilton Road Intersection Improvements FRA-16-6.87, PID No. 105768 CTL Project No.: 20050033COL

Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-002-0-19	08/19/2020	6.0"	719937.2300	1863258.5390	802.236



Depth (inches)	Pavement Type	Notes
0 - 6.0	Asphalt Concrete	Surface Course
6.0 - 12.0	Asphalt Concrete	Intermediate Course

Recovered Core Length	In-hole Depth	Recovery	
(inches)	(inches)	(%)	
12.0	12.0	100	



PAVEMENT CORE REPORT

Broad Street and Hamilton Road Intersection Improvements FRA-16-6.87, PID No. 105768 CTL Project No.: 20050033COL

Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-003-0-19	08/19/2020	6.0"	719950.6350	1863573.5860	803.144



Depth (inches)	Pavement Type	Notes	
0 - 7.0	Asphalt Concrete	Surface Course	
7.0 - 10.0	Asphalt Concrete	Intermediate Course	

Recovered Core Length	In-hole Depth	Recovery	
(inches)	(inches)	(%)	
10.0	10.0	100	



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-004-0-19	08/19/2020	6.0"	720024.8290	1863881.6160	804.244



Depth (inches)	Pavement Type	Notes
0 - 11.0	Asphalt Concrete	

Recovered Core Length	In-hole Depth	Recovery			
(inches)	(inches)	(%)			
7.0*	11.0	63.6			
* Portion of core crumbled during recovery					



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-005-0-19	08/19/2020	6.0"	720086.6100	1864283.5500	801.853



Depth (inches)	Pavement Type	Notes
0 - 9.0	Asphalt Concrete	

Recovered Core Length	In-hole Depth	Recovery			
(inches)	(inches)	(%)			
8.0*	9.0	88.9*			
* Portion of core crumbled during recovery					



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-006-0-19	08/19/2020	6.0"	720141.7380	1864716.6070	792.207



Depth (inches)	Pavement Type	Notes
0 - 11.0	Asphalt Concrete	

Recovered Core Length	In-hole Depth	Recovery	
(inches)	(inches)	(%)	
11.0	11.0	100	



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-007-0-19	08/19/2020	6.0"	719002.8020	1863519.9800	800.395



Depth (inches)	Pavement Type	Notes
0 - 2.0	Asphalt Concrete	Surface Course
2.0 - 12.5	Cement Concrete	

Recovered Core Length	In-hole Depth	Recovery			
(inches)	(inches)	(%)			
10.5*	12.5	84*			
* Portion of core crumbled and not recovered					



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-008-0-19	08/19/2020	6.0"	719386.9440	1863613.6140	800.023



Depth (inches)	Pavement Type	Notes	
0 - 5.5	Asphalt Concrete	Surface Course	
5.5 – 12.75	Cement Concrete		

Recovered Core Length	In-hole Depth	Recovery
(inches)	(inches)	(%)
12.75	12.75	100



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-008-1-19	08/19/2020	6.0"	719259.4230	1863583.7760	799.450



Depth (inches)	Pavement Type	Notes
0 - 5.0	Asphalt Concrete	Surface Course
5.0 – 11.0	Cement Concrete	Crumbled

Recovered Core Length	In-hole Depth	Recovery		
(inches)	(inches)	(%)		
5.0*	11.0	45.45*		
* Portion of core crumbled				



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-008-2-19	08/19/2020	6.0"	719258.5290	1863580.5380	799.356



Depth (inches)	Pavement Type	Notes
0 - 5.0	Asphalt Concrete	Surface Course
5.0 – 12.0	Cement Concrete	Crumbled

Recovered Core Length	In-hole Depth	Recovery			
(inches)	(inches)	(%)			
5.0*	12.0	41.67*			
* Portion of core crumbled					



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-009-0-19	08/19/2020	6.0"	719775.1940	1863706.5900	802.247

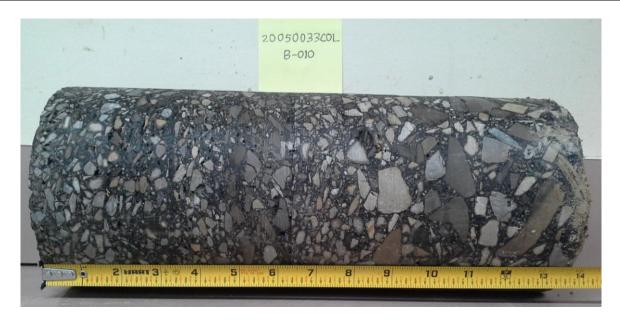


Depth (inches)	Depth (inches) Pavement Type	
0 - 4.0	Asphalt Concrete	Surface Course
4.0 - 8.5	Cement Concrete	
8.5 – 17.0	Cement Concrete	Crumbled (Not Included in the Picture)

Recovered Core Length	In-hole Depth	Recovery			
(inches)	(inches)	(%)			
8.5*	17.0	50*			
* Portion of core crumbled					



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-010-0-19	08/19/2020	6.0"	720323.1410	1863736.9400	804.328



Depth (inches)	Pavement Type	Notes
0 - 8.0	Asphalt Concrete	Surface Course
8.0 - 14.5	Asphalt Concrete	Intermediate Course

Recovered Core Length	In-hole Depth	Recovery				
(inches)	(inches)	(%)				
14.0*	14.5	96.5*				
* Portio	on of core crumbled and not	t recovered				



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-011-0-19	08/19/2020	6.0"	720679.7470	1863716.9080	805.303



Depth (inches)	Pavement Type	Notes
0 - 8.0	Asphalt Concrete	Surface Course
8.0 – 13.0	Asphalt Concrete	Intermediate Course

Recovered Core Length	In-hole Depth	Recovery
(inches)	(inches)	(%)
13.0	13.0	100



Core No.	Date Cored	Core Diameter	Northing (feet)	Easting (feet)	Elevation (feet)
B-012-0-19	08/19/2020	6.0"	721094.9430	1863801.1250	806.329



Depth (inches)	Pavement Type	Notes
0 - 5.5	Asphalt Concrete	Surface Course
5.5 – 18.0	Cement Concrete	

Recovered Core Length	In-hole Depth	Recovery
(inches)	(inches)	(%)
16.5	16.5	100



APPENDIX D GB1 ANALYSIS





OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

FRA-16-6.87 105768

Broad Street and Hamilton Road Intersection Improvements

CTL Engineering, Inc.

Prepared By:

SR

Date prepared:

Monday, October 12, 2020

Shahedur Rahman, E.I.

2860 Fisher Rd Columbus Ohio 43204 (614) 276-8123

srahman@ctleng.com

NO. OF BORINGS:

12





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-001-0-19	SR 16 (East Broad St)	58+79.7	7	Right	CME 45C Track	89	800.5	799.0	1.5 C
2	B-002-0-19	SR 16 (East Broad St)	63+18.6	10	Left	CME 45C Track	89	802.2	800.7	1.5 C
3	B-003-0-19	SR 16 (East Broad St)	56+32.54	20	Right	CME 45C Track	89	803.1	801.6	1.5 C
4	B-004-0-19	SR 16 (East Broad St)	59+47.82	11	Left	CME 45C Track	89	804.2	802.7	1.5 C
5	B-005-0-19	SR 16 (East Broad St)	73+54.34	20	Left	CME 45C Track	89	801.9	800.4	1.5 C
6	B-006-0-19	SR 16 (East Broad St)	77+90.88	17	Left	CME 45C Track	89	792.2	790.7	1.5 C
7	B-007-0-19	SR 317 (Hamilton Rd)	69+5.66	1	Right	CME 45C Track	89	800.4	798.9	1.5 C
8	B-008-0-19	SR 317 (Hamilton Rd)	73+3.33	24	Left	CME 45C Track	89	800.0	798.5	1.5 C
9	B-009-0-19	SR 317 (Hamilton Rd)	77+0.87	26	Right	CME 45C Track	89	802.2	800.7	1.5 C
10	B-010-0-19	SR 317 (Hamilton Rd)	32+49.63	21	Right	CME 45C Track	89	804.3	802.8	1.5 C
11	B-011-0-19	SR 317 (Hamilton Rd)	286+4.2	22	Left	CME 45C Track	89	805.3	803.8	1.5 C
12	B-012-0-19	SR 317 (Hamilton Rd)	90+26.49	1	Right	CME 45C Track	89	806.3	804.8	1.5 C

1/18/2019



	Boring	Sample		nple pth	_	rade pth		dard tration	НР		P	hysica	al Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate	Proble	m	Excavate ar	•	Recommendation
#			From		From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	Mc	M _{OPT}	Class	GI	Content (ppm)	Unsuitable	Unstable	Unsuitable		(Enter depth in inches)
1	В	SS-1	1.5	3.0	0.0	1.5	27		4	31	21	10	37	26	63	18	16	A-4a	6						
	001-0	SS-2	3.0	4.5	1.5	3.0	10		2.75	54	25	29	36	52	88	23	22	A-7-6	18			N ₆₀			
	19	SS-3	4.5	6.0	3.0	4.5	15		2.75							21	18	A-7-6	16						
		SS-4	6.0	7.5	4.5	6.0	10	10	0.5							32	18	A-7-6	16						
2	В	SS-1	1.0	2.5	-0.5	1.0	22			32	20	12	30	24	54	18	15	A-6a	5			Mc			
	002-0	SS-2	2.5	4.0	1.0	2.5	21		4.5	38	23	15	38	39	77	19	18	A-6a	10						
	19	SS-3	4.0	5.5	2.5	4.0	25		4.5							13	16	A-6b	16						
		SS-4	5.5	7.0	4.0	5.5	15	15	3							19	16	A-6b	16						
3	В	SS-1	1.0	2.5	-0.5	1.0	30			22	18	4	21	11	32	12	10	A-2-4	0						
	003-0	SS-2	2.5	4.0	1.0	2.5	10		2	48	28	20	41	48	89	27	25	A-7-6	14			N ₆₀		12"	
	19	SS-3	4.0	5.5	2.5	4.0	7		1.5							28	16	A-6b	16						
		SS-4	5.5	7.0	4.0	5.5	18	7	3.5							20	16	A-6b	16						
4	В	SS-1	2.0	3.5	0.5	2.0	10		3.75	37	22	15	47	34	81	22	17	A-6a	10			N ₆₀ & Mc		12"	
	004-0	SS-2	3.5	5.0	2.0	3.5	16									17	10	A-2-6	4			Mc			
	19	SS-3	5.0	6.5	3.5	5.0	25		4.5	44	21	23	41	42	83	23	18	A-7-6	14						
		SS-4	6.5	8.0	5.0	6.5	42	10	3.5							17	8	A-3	0						
5	В	SS-1	1.0	2.5	-0.5	1.1	57			21	17	4	21	11	32	12	10	A-2-4	0						
	005-0	SS-2	2.5	4.0	1.1	2.6	37		4.5	33	19	14	40	32	72	15	14	A-6a	9						
	19	SS-3	4.0	5.5	2.6	4.1	27		4.5							14	14	A-6a	10						
	-10	SS-4	5.5	7.0	4.1	5.6	28	27	4.5							14	14	A-6a	10						
6	В	SS-1	1.0	2.5	-0.5	1.0	73			23	17	6	19	11	30	8	10	A-2-4	0						
	006-0	SS-2	2.5	4.0	1.0	2.5	39		4.5	24	16	8	36	25	61	11	11	A-4a	5						
	19	SS-3	4.0	5.5	2.5	4.0	25		4.5							10	10	A-4a	8						
		SS-4	5.5	7.0	4.0	5.5	24	24	4.5							11	16	A-6b	16						
7	В	SS-1	1.0	2.5	-0.5	1.0	9			32	20	12	21	19	40	12	15	A-6a	2			N ₆₀		12"	
	007-0	SS-2	2.5	4.0	1.0	2.5	18		4.5	45		15	48	48	96	16		A-7-5	11		A-7-5		30"		
	19	SS-3	4.0	5.5	2.5	4.0	16									28	16	A-6b	16						
	13	SS-4	5.5	7.0	4.0	5.5	55	9	4.5							18	0	Rock	0						
8	В	SS-1	1.0	2.5	-0.5	1.0	21	,	5	19	15	4	17	11	28	9	8	A-3a	0						
	008-0	SS-2	2.5	4.0	1.0	2.5	15	1				NP		7	23	7	6	A-1-b	0						
	19	SS-3	4.0		2.5	3.6	-13	-				. * "	-10	,		10	6	A-1-b	0						
	13	SS-3 SS-4	4.0	5.1	2.5	3.0		15								10	0	A-1-0				<u> </u>			
9	В	SS-1	1.5	3.0	0.0	1.5	10	13	1.25	35	21	14	47	39	86	22	16	A-6a	10			HP & Mc		12"	
	009-0	SS-2	3.0	4.5	1.5	3.0	9		1.25					53	91	25	24	A-7-6	20			HP			
								-		00	21	JJ	36	33	31							HE			
	19	SS-3	4.5	6.0	3.0	4.5	18		1.5							29	18	A-7-6	16						

1/18/2019



#	Boring	Sample	Sam De	•	1 ~	rade pth		dard	НР		Pl	hysic	al Chara	cteristics		Mo	isture	Ohio	DOT	Sulfate Content	Proble	m	Excavate an	•	Recommendation (Enter depth in
			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI	(ppm)	Unsuitable	Unsuitable Unstable	Unsuitable	Unstable	inchas)
		SS-4	6.0	7.5	4.5	6.0	15	9	1							29	18	A-7-6	16						
10	В	SS-1	1.5	3.0	0.0	1.5	18		1.75	39	24	15	42	37	79	21	19	A-6a	10						
	010-0	SS-2	3.0	4.5	1.5	3.0	12		2.5	56	26	30	38	51	89	26	23	A-7-6	19			N ₆₀ & Mc			
	19	SS-3	4.5	6.0	3.0	4.5	15		4.5							22	18	A-7-6	16						
		SS-4	6.0	7.5	4.5	6.0	21	12	2.5							20	18	A-7-6	16						
11	В	SS-1	1.5	3.0	0.0	1.5	10		2.25	37	18	19	47	37	84	23	16	A-6b	12			N ₆₀ & Mc		12"	
	011-0	SS-2	3.0	4.5	1.5	3.0	12		1.75	51	26	25	53	42	95	27	23	A-7-6	16			HP & Mc			
	19	SS-3	4.5	6.0	3.0	4.5	13		3.25							25	16	A-6b	16						
		SS-4	6.0	7.5	4.5	6.0	21	10	1.5							20	16	A-6b	16						
12	В	SS-1	1.5	3.0	0.0	1.5	13									5	10	A-2-6	4						
	012-0	SS-2	3.0	4.5	1.5	3.0	36		4.5	30	19	11	37	33	70	13	14	A-6a	7						
	19	SS-3	4.5	6.0	3.0	4.5	27		4.5							18	14	A-6a	10						
		SS-4	6.0	7.5	4.5	6.0	43	13	4.5							13	14	A-6a	10						



PID: 105768

County-Route-Section: FRA-16-6.87

No. of Borings: 12

Geotechnical Consultant: CTL Engineering, Inc.

Prepared By: SR

Date prepared: 10/12/2020

(Chemical Stabilization Options				
320	Rubblize & Roll	Option			
206	Cement Stabilization	Option			
	Lime Stabilization	No			
206	Depth	12"			

Excavate and Replace					
ons					
18"					
24"					
Global Geogrid					
Override(N60L): 12"					
Override(HP): 18"					

Design CBR	6
---------------	---

% Samples within 6 feet of subgrade							
N ₆₀ ≤ 5	0%	HP ≤ 0.5	2%				
N ₆₀ < 12	19%	0.5 < HP ≤ 1	2%				
12 ≤ N ₆₀ < 15	9%	1 < HP ≤ 2	17%				
N ₆₀ ≥ 20	47%	HP > 2	55%				
M+	15%						
Rock	0%						
Unsuitable	4%		·				

Excavate and Replace at Surface					
Average	0"				
Maximum	0''				
Minimum	0"				

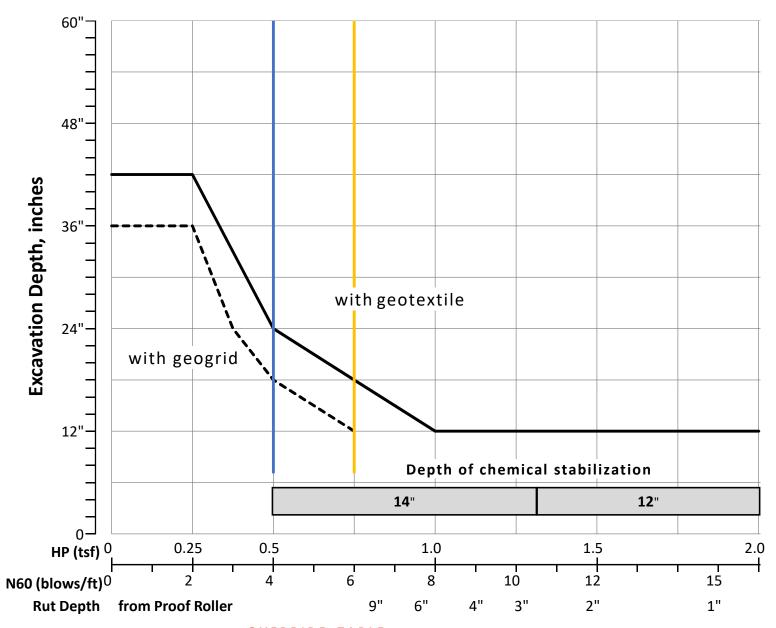
% Proposed Subgrade Surface				
Unstable & Unsuitable	40%			
Unstable	37%			
Unsuitable	3%			

	N ₆₀	N _{60L}	НР	LL	PL	PI	Silt	Clay	P 200	M _c	M _{OPT}	GI
Average	23	13	3.20	37	22	15	35	32	67	18	15	10
Maximum	73	27	4.50	60	30	33	53	53	96	32	25	20
Minimum	7	7	0.50	19	15	4	16	7	23	5	0	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	1	0	2	3	0	2	0	1	1	3	0	0	12	9	1	12	0	0	47
Percent	2%	0%	4%	6%	0%	4%	0%	2%	2%	6%	0%	0%	26%	19%	2%	26%	0%	0%	100%
% Rock Granular Cohesive	2%					26%								72	2%				100%
Surface Class Count	0	0	2	3	0	2	0	0	1	3	0	0	9	4	1	5	0	0	30
Surface Class Percent	0%	0%	7%	10%	0%	7%	0%	0%	3%	10%	0%	0%	30%	13%	3%	17%	0%	0%	100%

1/18/2019

GB1 Figure B – Subgrade Stabilization



OVERRIDE TABLE

Calculated Average	New Values	Check to Override
3.20	0.50	✓ HP
13.42	6.00	✓ N60L

Average HP Average N_{60L}



The subgrade analysis workbook consists of five worksheets. Each worksheet functions independently. In all of the worksheets the fields are color coded as follows:

- Every yellow highlighted field indicates a field to be entered by the user.
- Every salmon field is to indicate a problem/issue.
- Every gray or green field is a heading/informational field.

IMPORTANT: The sequence of filling out the data needs to be followed as outlined below:

Cover Sheet: this worksheet is designed for the purpose of entering the project information.
 Enter all the following fields:

County-Route-Section	This includes the county, route, section number assigned to the project.
PID	the Project Identification Number
Project Description	See Cover Sheet for list of example details
Geotechnical Consultant	The Geotechnical Consultant performing the analysis.
Prepared By	The preparer of the subgrade analysis
Date prepared	The date the analysis is performed.
Contact Information	Name, address, telephone #, and email address
No. of Borings	Enter the total number of borings within the alignment that is being analyzed.

- 2. Boring Logs Entry Worksheet: this worksheet has a programming code that will run in the background every time the sheet is activated and will make the sheet unresponsive for less than a minute. The code is designed to read the total number of borings from the cover sheet and generate the needed number of fields.
 - a. All yellow highlighted fields are user's entry.
 - b. ODOT has developed a text table export from gINT (GB 1 Borings Log Entry Tab) that will allow for copy and paste of all highlighted fields with the exception of proposed subgrade elevation. The designer must provide a proposed subgrade elevation in order for the spreadsheet to function properly.
 - c. The Cut/Fill field is a calculated field that, based on the difference between the boring elevation and the proposed subgrade elevation, will highlight the cell either gray and adds the letter "C" to the end in a cut situation or highlights the cell in light purple and adds the letter "F" to the end in a fill situation.
 - d. Every duplicate boring ID will be highlighted in salmon background and red text.
 - e. **IMPORTANT**: After entering all the borings' information, the user must click "Add Subgrade Analysis Entry Fields" button. This will generate all the required fields in the "Subgrade Analysis" Worksheet.
- 3. Subgrade Analysis Worksheet:
 - a. The boring number and boring ID is read from the "Boring Logs Entry Worksheet" excluding every boring that has six feet or more of fill.
 - b. All yellow highlighted fields are to be entered by the user and salmon highlighted fields indicates a problem or issue.
 - c. Every sample that has a Sulfate Content greater than or equal to 3000 will be highlighted in light salmon background. Every sample that has a Sulfate Content greater than or equal to 8000 will be highlighted in darker salmon background. Note the revised sulfate criteria in GB1 issued July 20, 2018.



d. Unsuitable/Unstable:

- i. Unsuitable samples that are within 3 feet of the top of subgrade will be highlighted with salmon background and the class will be showing in this field.
- ii. Unstable Samples that are within 3 feet of top of subgrade will be highlighted with salmon background and text to indicate the problem as follows:

Criterion	Stabilization Need Check	Text displayed in the field
A-1-a, A-1-b, A-3, or A-3a Soil Class	No Stabilization is needed	
HP ≥ 1.875	No Stabilization is needed	
N ₆₀ ≥ 15	No Stabilization is needed	
$1.875 \ge HP \ge 1.5$ and M _c ≥ Opt. M _c +3	Unstable Subgrade	HP & Mc
15 ≥ N_{60} ≥ 12 and M_c ≥ Opt. M_c +3	Unstable Subgrade	N ₆₀ & Mc
HP ≤ 1.5	Unstable Subgrade	HP
N ₆₀ ≤ 12	Unstable Subgrade	N ₆₀

- iii. The field is formulated to check for HP first and check for N₆₀ second.
- e. Excavate and Replace (Item 204) is going to be calculated based on the subgrade depth for each sample indicating an unsuitable or unstable problem.
- f. Recommendation:
 - i. Geotextile Option is calculated and rounded to a multiple of 3 inches based on the subgrade depth for every sample indicating an unsuitable or unstable problem.
 - ii. GEOGRID Option is only offered in case of unstable subgrade problem and if the geotextile option indicates the need to excavate greater than 12 inches.

PLEASE NOTE: The Problem, Excavate & Replace, and Recommendation Fields are the responsibility of the Designer. These fields are being enhanced to attempt to capture the ODOT philosophy regarding the GB1 stabilization chart, but are considered still under development. If there are discrepancies between the spreadsheet output and the GB1 chart - the chart governs in conjunction with engineering judgement. Please contact Steve Taliaferro at stephen.taliaferro@dot.ohio.gov if you have any questions.

PLEASE NOTE: It is the Designer's responsibility to identify the most representative data when samples have been separated into multiple specimen (say 1.5 to 2.3 feet and 2.3 to 3.0 feet). The spreadsheet is not capable at this time of addressing this issue within a direct data export from gINT.

4. Results Summary:

All fields in this sheet are password protected and are either calculated or read from the other worksheets.

Graph Worksheet:

This worksheet is designed to read the average N_{60L} and the average HP from the Cover Sheet and plot a blue line for Average HP and orange line for Average N_{60L} on GB1 Figure B – Subgrade Stabilization. The Override Table can be used to enter HP and/or N_{60L} values that are different than the calculated averages. The Override values will change the global undercut recommendation in the Results Summary.