LUC-23 - BrR Load Rating Background Calcs

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Date: 8/24/2022

Checker: F. Getz

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This sheet is give the load rating assumptions and inputs.

This sheet is in accordance with AISC Manual for Steel Construction, 15th Ed.

User inputs are highlighted in - Yellow

Sheet assumption are highlighted in -

Blue

Sheet checks are highlighted in -

Green

Inputs from other files are highlighted in purple -

Purple

Diaphragm Weights

1958 Interior Diaphragm

$$Angle_{wt} := 6.1 \cdot plf$$

Angle_{Length} :=
$$(7 \cdot \text{ft} + 3 \cdot \text{in}) + 2 \cdot \left[(36 \cdot \text{in})^2 + (7 \cdot \text{ft} + 3 \cdot \text{in})^2 \right]^{0.5} = 22.942 \text{ ft}$$

$$Int_{1958 \ wt} := Angle_{wt} \cdot Angle_{Length} = 0.14 \cdot kip$$

1958 End Diaphragm

Angle Length:
$$= \left(\frac{7 \cdot \text{ft} + 3 \cdot \text{in}}{\cos(3.175 \cdot \text{deg})}\right) + 2 \cdot \left[(36 \cdot \text{in})^2 + \left(\frac{1}{2} \cdot \frac{7 \cdot \text{ft} + 3 \cdot \text{in}}{\cos(3.175 \cdot \text{deg})}\right)^2 \right]^{0.5} = 16.681 \text{ ft}$$

$$\operatorname{End}_{1958_{\text{wt}}} := \operatorname{Angle}_{\text{wt}} \cdot \operatorname{Angle}_{\text{Length}} + \left(\frac{3}{8} \cdot \operatorname{in}\right) \cdot (8 \cdot \operatorname{in}) \cdot (12 \cdot \operatorname{in}) \cdot 490 \cdot \operatorname{pcf} = 0.147 \cdot \operatorname{kip}$$

2022 End Diaphragm

$$Channel_{wt} := 42.7 \cdot plf$$

Channel_{Length} :=
$$\frac{7 \cdot \text{ft} + 3 \cdot \text{in}}{\cos(3.175 \cdot \text{deg})} = 7.261 \text{ ft}$$

Stiffener_vol :=
$$\left(\frac{1}{2} \cdot in\right) \cdot \left(8 \cdot \frac{1}{4} \cdot in\right) \cdot (34 \cdot in)$$

$$\mathsf{End}_{2022\ wt} \coloneqq \mathsf{Channel}_{wt} \cdot \mathsf{Channel}_{Length} + 2 \cdot \mathsf{Stiffener_vol} \cdot 490 \cdot \mathsf{pcf} = 0.39 \cdot \mathsf{kip}$$

2022 Interior Diaphragm

Angle Length:
$$= (6 \cdot \text{ft} + 0 \cdot \text{in}) + 2 \cdot \left[(36 \cdot \text{in})^2 + (6 \cdot \text{ft} + 0 \cdot \text{in})^2 \right]^{0.5} = 19.416 \,\text{ft}$$

Stiffener vol :=
$$\left(\frac{3}{8} \cdot \text{in}\right) \cdot (7.87 \cdot \text{in}) \cdot (34 \cdot \text{in})$$

Filler_PL_Vol :=
$$\left(\frac{3}{8} \cdot \text{in}\right) \cdot (12 \cdot \text{in}) \cdot (4 \cdot \text{in})$$

$$Int_{2022~wt} \coloneqq Angle_{wt} \cdot Angle_{Length} + (2 \cdot Stiffener_vol + Filler_PL_Vol) \cdot 490 \cdot pcf = 0.227 \cdot kiptoner$$

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Checker: R. Brinkman

Date: 6/29/2023

$$\gamma_{conc} := 150 \cdot pcf$$

$$\gamma_{\text{steel}} := 490 \cdot \text{pcf}$$

Total Span Length := $2 \cdot (52 \cdot \text{ft} + 86.5 \cdot \text{ft}) = 277 \text{ ft}$

Program defines the loads from CL to CL of Bearings So use this for determine loads

ODOT Standard Drawing - BR-2-15

BR-2-15

 $HSS4x3x0_25_{wt} := 10.48 \cdot plf$

 $HSS4x4x0_188_{wt} := 9.40 \cdot plf$

 $Post_{height} := 1 \cdot ft + 6 \cdot in - 0.75 \cdot in = 17.25 \cdot in$

BR_Base_Plate_{wt} := $(10.5 \cdot \text{in} \cdot 10.5 \cdot \text{in} \cdot 0.75 \cdot \text{in}) \cdot \gamma_{\text{steel}} = 0.023 \cdot \text{kip}$

Hardware_Increase := 105.%

Increase by 5% to account for bolts splice plates, etc.

Total Posts South := $(24) \cdot 2 = 48$

$$South_BR_{wt_total} := \begin{bmatrix} 2 \cdot HSS4x3x0_25_{wt} \dots \\ + \begin{pmatrix} HSS4x4x0_188_{wt} \cdot Post_{height} \dots \\ + BR_Base_Plate_{wt} \end{pmatrix} \cdot \frac{Total_Posts_South}{Total_Span_Length} \end{bmatrix} \cdot Hardware_Increase = 0.029 \cdot klf$$

BR-2-15 - Modified

The middle barrier has tubes on both sides

 $Backing_Plate_{wt} := (8 \cdot in \cdot 3 \cdot in \cdot 0.25 \cdot in) \cdot \gamma_{steel} = 0.002 \cdot kip$

Total Posts Middle := $(22) \cdot 2 = 44$

$$\begin{aligned} \text{Middle_BR}_{wt_total} \coloneqq \begin{bmatrix} 4 \cdot \text{HSS4x3x0_25}_{wt} \dots \\ + \begin{pmatrix} \text{HSS4x4x0_188}_{wt} \cdot \text{Post}_{\text{height}} \dots \\ + \text{BR_Base_Plate}_{wt} + 2 \cdot \text{Backing_Plate}_{wt} \end{bmatrix} \cdot \frac{\text{Total_Posts_South}}{\text{Total_Span_Length}} \end{aligned} \right] \cdot \\ \begin{aligned} \text{Hardware_Increase} &= 0.051 \cdot \text{klf} \end{aligned}$$

Fence

$$Rail_{wt} := 7.93 \cdot plf$$

HSS 3.5 x 2 x 1/4

$$Post_{wt} := 17.20 \cdot plf$$

HSS 3.5 x 2 x 3/8

Post Ht :=
$$6 \cdot \text{ft} - 0.625 \cdot \text{in} = 5.948 \text{ ft}$$

$$Picket_{wt} := 1 \cdot in \cdot 1 \cdot in \cdot \gamma_{steel} = 3.403 \cdot plf$$

$$Picket_{ht} := 4ft + 6in$$

Approximate by dividing the max spacing by the total span length. Some pickets will be closer spaced, and there are pilasters but this is a good approximation

No Fence Posts :=
$$(20) \cdot 2 = 40$$

Fence_Base_Plate_{wt} :=
$$(8 \cdot \text{in} \cdot 9 \cdot \text{in} \cdot 0.625 \cdot \text{in}) \cdot \gamma_{\text{steel}}$$

Same for both North and South

$Mesh_{wt} := 2 \cdot psf$

$$Mesh_{ht} := Picket_{ht} = 4.5 ft$$

$$\begin{aligned} & \text{Total_Fence}_{wt_total} := \begin{bmatrix} 3 \cdot \text{Rail}_{wt} + \begin{pmatrix} \text{Post}_{wt} \cdot \text{Post_Ht} & ... \\ + \text{Fence_Base_Plate}_{wt} \end{pmatrix} \cdot \frac{\text{No_Fence_Posts}}{\text{Total_Span_Length}} & ... \end{bmatrix} \\ & + \begin{pmatrix} \text{Picket}_{wt} \cdot \text{Picket}_{ht} \end{pmatrix} \cdot \frac{\text{No_Pickets}}{\text{Total_Span_Length}} & ... \\ & + \begin{pmatrix} \text{Mesh}_{wt} \cdot \text{Mesh}_{ht} \end{pmatrix} \cdot \frac{\text{No_Pickets}}{\text{Total_Span_Length}} & ... \end{bmatrix} \end{aligned}$$

Total South Barrier

South_Conc_Barrier_{wt} :=
$$[(1.5 \cdot \text{ft} \cdot 1.5 \cdot \text{ft}) + 6 \cdot \text{in} \cdot (1 \cdot \text{ft} + 8 \cdot \text{in})] \cdot \gamma_{\text{conc}} = 0.462 \cdot \text{klf}$$

$$South_Total_Barrier_{wt} := South_Conc_Barrier_{wt} + South_BR_{wt_total} + Total_Fence_{wt_total} = 0.591 \cdot klf$$

$$South_Program_Conc_{wt} := 0.450 \cdot klf$$

There are limitations in the program as to how to input the loads, take the program load for the concrete and then adjust the superimposed load as needed to get the total load calculated above

$$South_Add_Load_Program_Input_{wt} := South_Total_Barrier_{wt} - South_Program_Conc_{wt} = 0.141 \cdot klf$$

Total Middle Barrier

Middle_Conc_Barrier_{wt} :=
$$[2 \cdot \text{ft} \cdot (1 \cdot \text{ft} + 1.5 \cdot \text{in})] \cdot \gamma_{\text{conc}} = 0.337 \cdot \text{klf}$$

$$Middle_Total_Barrier_{wt} := Middle_Conc_Barrier_{wt} + Middle_BR_{wt_total} = 0.389 \cdot klf$$

Total North Barrier

$$North_Conc_Barrier_{wt} := [[1.5 \cdot ft \cdot (1 \cdot ft + 1.5 \cdot in)] + 6 \cdot in \cdot (1 \cdot ft + 3.5 \cdot in)] \cdot \gamma_{conc} = 0.35 \cdot klf$$

 $North_Total_Barrier_{wt} := North_Conc_Barrier_{wt} + Total_Fence_{wt} \ total = 0.45 \cdot klf$

 $North_Program_Conc_{wt} := 0.338 \cdot klf$

There are limitations in the program as to how to input the loads, take the program load for the concrete and then adjust the superimposed load as needed to get the total load calculated above

 $North_Add_Load_Program_Input_{wt} := North_Total_Barrier_{wt} - North_Program_Conc_{wt} = 0.112 \cdot klf$

Pilaster Loads

Abutment Pilaster Length := $5 \cdot \text{ft} + 5 \cdot \text{in}$

Interior Pilaster Length := 3.ft

South Pilaster Width := $1 \cdot \text{ft} + 3.5 \cdot \text{in}$

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\begin{aligned} \text{South\_Abut\_Pilaster\_Load} \coloneqq & \text{Abutment\_Pilaster\_Length} \cdot \left[ (6 \cdot \text{ft} + 1 \cdot \text{in}) \text{South\_Pilaster\_Width} \gamma_{\text{conc}} \right] \dots \\ & + (\text{Abutment\_Pilaster\_Length} + 4 \cdot \text{in}) \cdot \left[ (7.5 \cdot \text{in}) \cdot (\text{South\_Pilaster\_Width} + 3 \cdot \text{in}) \cdot \gamma_{\text{conc}} \right] \dots \\ & + & \text{Abutment\_Pilaster\_Length} \cdot (1 \cdot \text{ft} + 3.5 \cdot \text{in}) \cdot (2 \cdot \text{ft}) \cdot \gamma_{\text{conc}} \end{aligned}
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$$South_Interior_Pilaster_Load := Interior_Pilaster_Length \cdot \left[(6 \cdot ft + 1 \cdot in) South_Pilaster_Width \gamma_{conc} \right] \dots \\ + (Abutment_Pilaster_Length + 4 \cdot in) \cdot (7.5 \cdot in) \cdot (South_Pilaster_Width + 3 \cdot in) \cdot \gamma_{conc} \dots \\ + Abutment_Pilaster_Length \cdot (1 \cdot ft + 3.5 \cdot in) \cdot (2 \cdot ft) \cdot \gamma_{conc}$$

North Pilaster Width := 1.ft

$$\begin{aligned} \text{North_Abut_Pilaster_Load} \coloneqq & \text{Abutment_Pilaster_Length} \cdot \left[(6 \cdot \text{ft} + 1 \cdot \text{in}) \text{North_Pilaster_Width} \gamma_{\text{conc}} \right] \dots \\ & + (\text{Abutment_Pilaster_Length} + 4 \cdot \text{in}) \cdot \left[(7.5 \cdot \text{in}) \cdot (\text{North_Pilaster_Width} + 3 \cdot \text{in}) \cdot \gamma_{\text{conc}} \right] \dots \\ & + \text{Abutment_Pilaster_Length} \cdot (1 \cdot \text{ft} + 8 \cdot \text{in}) \cdot (2 \cdot \text{ft}) \cdot \gamma_{\text{conc}} \end{aligned}$$

North_Interior_Pilaster_Load := Interior_Pilaster_Length·
$$[(6 \cdot \text{ft} + 1 \cdot \text{in}) \text{North}_Pilaster_Width} \gamma_{conc}] \dots = 6.12 \cdot \text{kip} + (\text{Abutment}_Pilaster_Length} + 4 \cdot \text{in}) \cdot (7.5 \cdot \text{in}) \cdot (\text{North}_Pilaster_Width} + 3 \cdot \text{in}) \cdot \gamma_{conc} \dots + \text{Abutment}_Pilaster_Length} \cdot (1 \cdot \text{ft} + 8 \cdot \text{in}) \cdot (2 \cdot \text{ft}) \cdot \gamma_{conc}$$

Note that the barrier load MUST be defined the full length. The The pilaster weight calculated was the total weight. Therefore subtract off the barrier weight to be used to pinput into the BrR Program

G14

 $BrR_South_Abut_Pilaster_Load := South_Abut_Pilaster_Load - South_Total_Barrier_{wt} \cdot Abutment_Pilaster_Length = 6.111 \cdot kip$ $BrR_South_Interior_Pilaster_Load := South_Interior_Pilaster_Load - South_Total_Barrier_{wt} \cdot Abutment_Pilaster_Length = 3.263 \cdot kip$

G1

BrR_North_Abut_Pilaster_Load := North_Abut_Pilaster_Load - North_Total_Barrier_wt · Abutment_Pilaster_Length = 5.887 · kip

 $BrR_North_Interior_Pilaster_Load := North_Interior_Pilaster_Load - North_Total_Barrier_{wt} \cdot Abutment_Pilaster_Length = 3.682 \cdot kip$

Abut_Pil_Loc₁ := Abutment_Pilaster_Length $\div 2 - (2 \cdot \text{ft} + 9 \cdot \text{in} - 6 \cdot \text{in}) = 0.458 \cdot \text{ft}$

Abut_Pil_Loc₂ := Total_Span_Length - Abut_Pil_Loc₁ = 276.542 ft

Int Pil Loc $1 := 52 \cdot \text{ft}$

Int Pil Loc 2 := Int Pil Loc $1 + 57 \cdot \text{ft} + 8 \cdot \text{in} = 109.667 \text{ ft}$

Int Pil Loc 3 := Total Span Length – Int Pil Loc 2 = 167.333 ft

Int_Pil_Loc_4 := Total_Span_Length - Int_Pil_Loc_1 = 225 ft

Lighting Load

$$\begin{aligned} & \text{Lighting_Bump_Out}_{\text{Wt}} \coloneqq \begin{bmatrix} 1.5 \cdot \text{ft} \cdot (1 \cdot \text{ft} + 7 \cdot \text{in}) & \dots \\ & + 2 \cdot (1 \cdot \text{ft} + 5 \cdot \text{in}) \cdot (1 \cdot \text{ft} + 7 \cdot \text{in}) \end{bmatrix} \cdot 1.5 \cdot \text{ft} \cdot \gamma_{conc} = 1.544 \cdot \text{kip} \end{aligned}$$

 $Light_Pole_{wt} := 150 \cdot lbf$

 $Total_Light_{wt} := Lighting_Bump_Out_{wt} + Light_Pole_{wt} = 1.694 \cdot kip$

Light Loc $1 := 26 \cdot \text{ft} + 10 \cdot \text{in} = 26.833 \text{ ft}$

Light Loc $2 := 79 \cdot \text{ft} + 4 \cdot \text{in} = 79.333 \text{ ft}$

Light Loc $3 := 138 \cdot \text{ft} + 6 \cdot \text{in} = 138.50 \text{ ft}$

Light_Loc_4 := Total_Span_Length - Light_Loc_2 = 197.667 ft

Light Loc 5 := Total Span Length – Light Loc 1 = 250.167 ft

Bar Deck Offset Location

Bot_Bar_Dist :=
$$1.5$$
in + $\frac{5}{8}$ ·in + $\frac{1}{2}$ · $\frac{5}{8}$ in = 2.438 ·in

Top_Bar_Dist :=
$$2.5 \cdot in + \frac{1}{2} \cdot \frac{4}{8} \cdot in = 2.750 \cdot in$$

Pier_Bar_Dist :=
$$2.5 \cdot \text{in} + \frac{1}{2} \cdot \frac{5}{8} \cdot \text{in} = 2.813 \cdot \text{in}$$