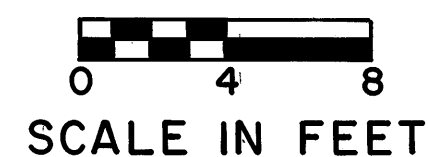
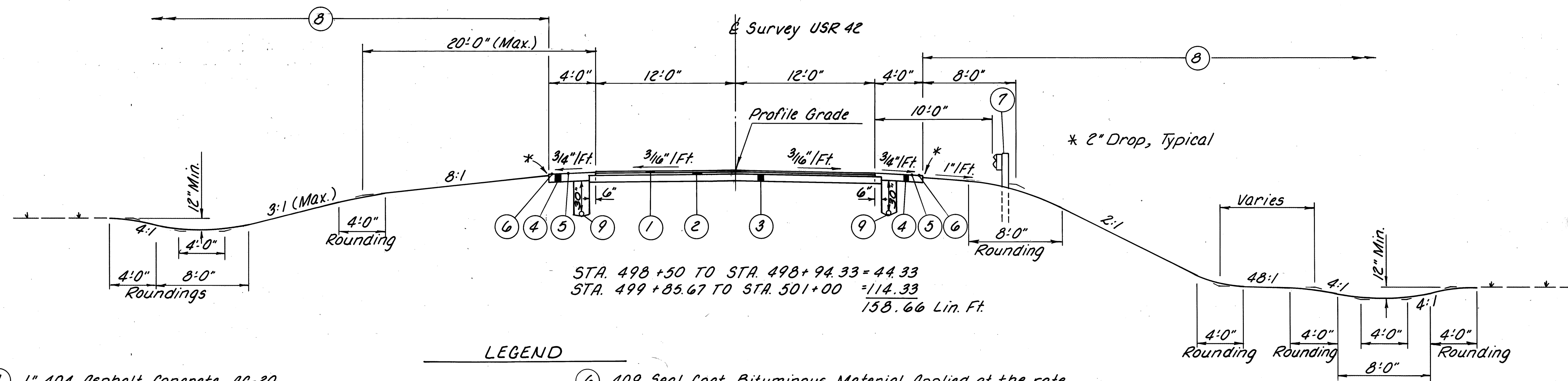


TYPICAL SECTIONS

TYPE 404 ON 301



CALC. BY: <u>DAV</u> DATE: <u>4-6-81</u> CHKD. BY: _____ DATE: _____	MED - 42 - 9.44	OHIO FHWA REGION 5	2 15
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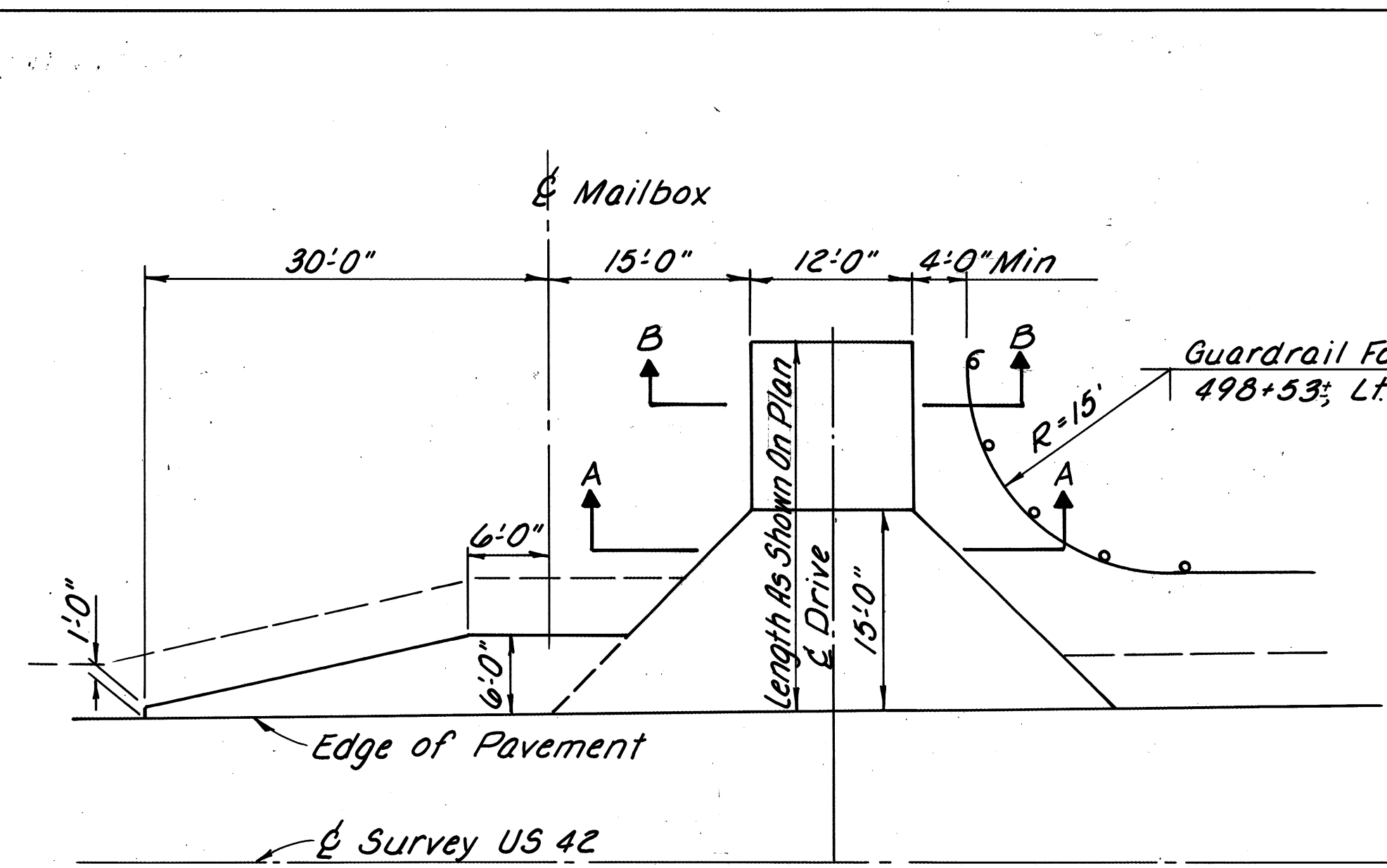


LEGEND

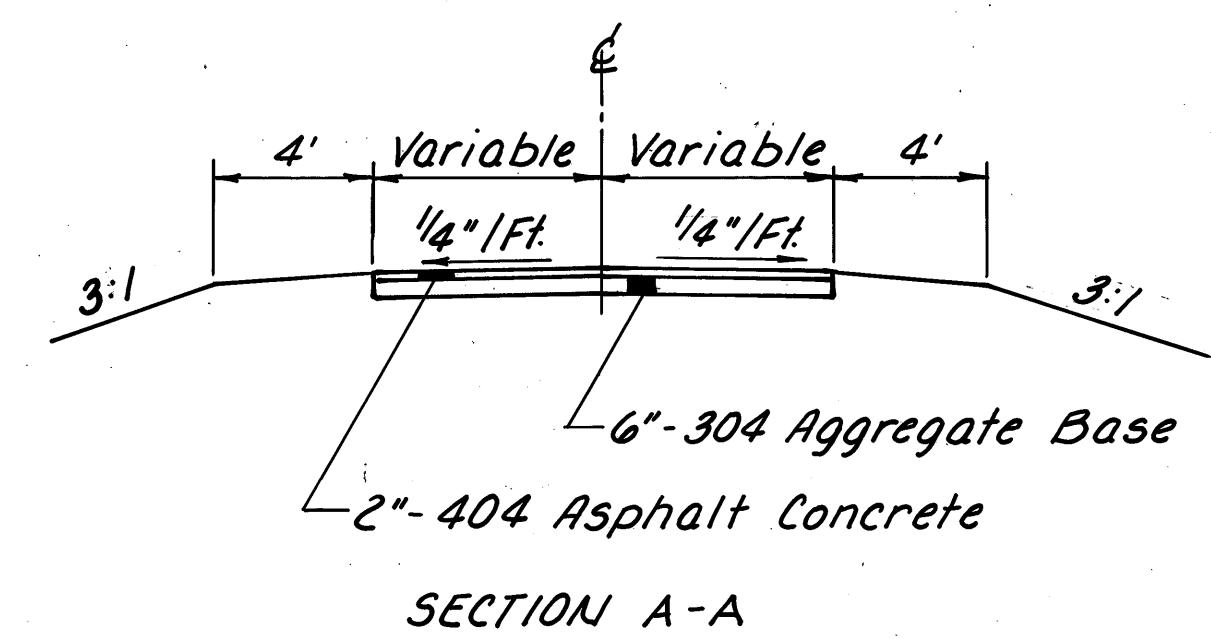
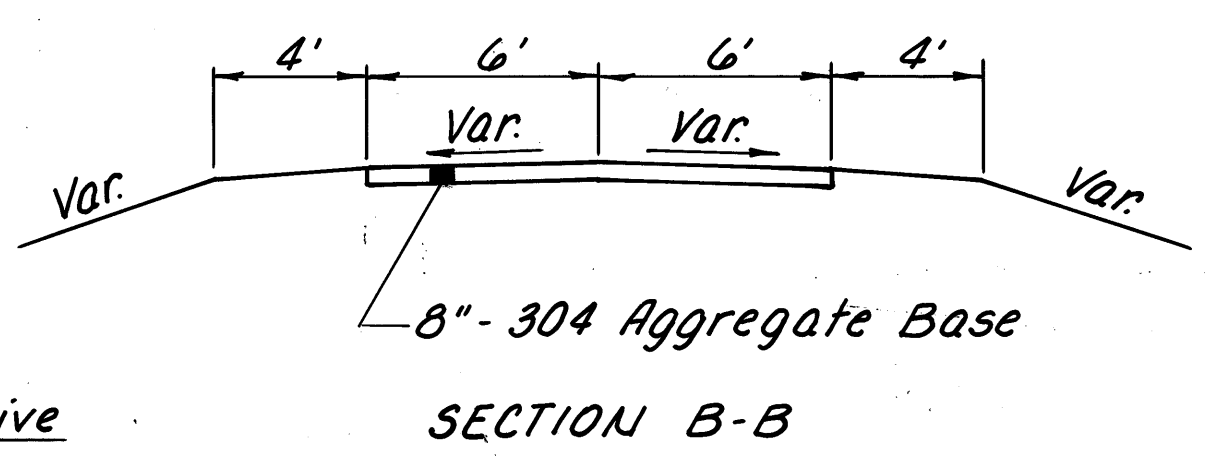
- ① 1" 404 Asphalt Concrete AC-20
- ② 1 1/2" 402 Asphalt Concrete AC-20
- ③ 6" 301 Bituminous Aggregate Base AC-20, RT-11 or RT-12
- ④ 8" 304 Aggregate Base
- ⑤ 408 Bituminous Prime Coat Applied at the rate of 0.40 Gal. Per Sq. Yd.
- ⑥ 409 Seal Coat Bituminous Material Applied at the rate of 0.30 Gal. Per Sq. Yd. and Cover Aggregate No. 8 Applied at the rate of 0.008 Cu. Yd. Per Sq. Yd.
- ⑦ 606 Guard Rail, Type 5
- ⑧ 659 Seeding and Mulching
- ⑨ 605 6" Shallow Pipe Underdrain

TAPER SECTIONS

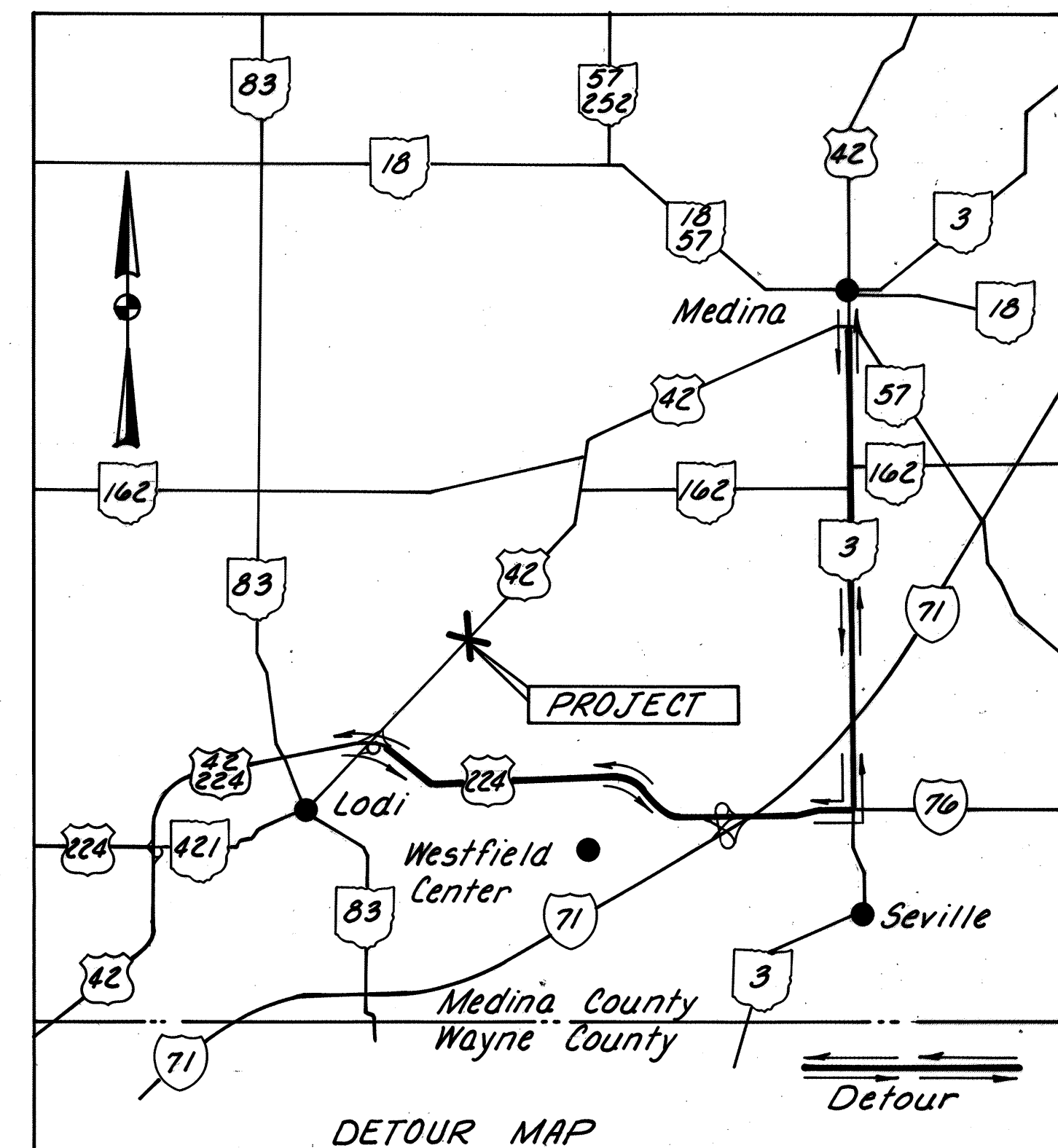
Sta. 497+50 To Sta. 498+50 (20' To 24')
 Sta. 501+00 To Sta. 502+00 (24' To 20')



TYPICAL DRIVE PLAN



TYPICAL DRIVE SECTIONS



DETOUR MAP

GENERAL NOTES

DUST CONTROL

THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED IN THE GENERAL SUMMARY TO BE USED FOR DUST CONTROL AS DIRECTED BY THE ENGINEER.

ITEM 616 WATER	50 M-GAL.
ITEM 616 CALCIUM CHLORIDE	0.5 TON

REMOVAL OF TREES AND STUMPS

ALL TREES AND STUMPS SPECIFICALLY MARKED FOR REMOVAL WITHIN THE CONSTRUCTION LIMITS OF THIS PROJECT SHALL BE REMOVED UNDER THE LUMP SUM PRICE BID FOR ITEM 201, CLEARING AND GRUBBING, EXCEPT THAT THOSE TREES FOR WHICH PROTECTION AND PRESERVATION WORK IS INDICATED ELSEWHERE IN THESE PLANS SHALL NOT BE REMOVED.

THE FOLLOWING IS AN APPROXIMATE ESTIMATE OF THE NUMBER OF TREES AND STUMPS TO BE REMOVED:

SIZES	NO. TREES	NO. STUMPS	TOTAL
18"	7	0	7

THE ABOVE ESTIMATE IS APPROXIMATE AND THE STATE OF OHIO RESERVES THE RIGHT TO ORDER THE REMOVAL OF ADDITIONAL TREES OR STUMPS OUTSIDE OF THE LIMITS OF CONSTRUCTION BUT WITHIN THE RIGHT-OF-WAY AND/OR EASEMENT LINES. PAYMENT FOR THE REMOVAL OF THESE ADDITIONAL TREES OR STUMPS SHALL BE INCLUDED IN THE LUMP SUM PRICE BID FOR ITEM 201, CLEARING AND GRUBBING.

SEEDING

QUANTITIES FOR SEEDING ARE CALCULATED FOR THE SOIL AREAS BETWEEN TEN (10) FEET OUTSIDE THE WORK LIMITS, AS SHOWN ON THE CROSS SECTIONS, OR TO THE RIGHT-OF-WAY LINE, IF SUCH LINE IS LESS THAN TEN (10) FEET FROM THE WORK LIMITS.

WATERING PERMANENT SEEDED AREAS

THE FOLLOWING ESTIMATED QUANTITIES ARE TO BE USED AS DIRECTED BY THE ENGINEER TO PROMOTE GROWTH AND TO CARE FOR THE PERMANENT SEEDED AREAS, AS PER 659.09:

ITEM 659 WATER	4 M-GAL.
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REMOVAL OF EXISTING PIPE

THE REMOVAL OF ALL EXISTING PIPE DRAINS, WHICH WOULD NORMALLY BE REMOVED IN VARIOUS EXCAVATION ITEMS, SHALL BE INCLUDED FOR PAYMENT IN THE UNIT PRICE BID FOR THE RESPECTIVE EXCAVATION ITEMS, UNLESS OTHERWISE ITEMIZED IN THE PLANS.

CONNECTIONS TO EXISTING PIPE

AT PLACES WHERE THE PLANS PROVIDE FOR PROPOSED PIPE TO BE CONNECTED TO EXISTING PIPE, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE THE EXISTING PIPE BOTH AS TO LINE AND TO GRADE BEFORE HE STARTS TO LAY THE PROPOSED PIPE. THE COST OF THIS OPERATION SHALL BE INCLUDED IN THE UNIT PRICE BID FOR THE PERTINENT 603 CONDUIT ITEMS.

EROSION CONTROL

ITEMS 601 AND 660 ARE PROVIDED IN THE PLANS FOR EROSION CONTROL. ROCK OR TURF OF A STABLE NATURE WILL NOT BE REMOVED IN ORDER TO PLACE ANY OF THESE ITEMS, AND TURF OF A STABLE NATURE WILL NOT BE REMOVED IN ORDER TO PLACE 660. THE ENGINEER SHALL CHECK AND NON-PERFORM QUANTITIES OR ADJUST LOCATIONS AND QUANTITIES FOR THESE ITEMS WHERE INDICATED BY FIELD CONDITIONS DURING CONSTRUCTION.

WATER POLLUTION, SOIL EROSION AND SILTATION CONTROL

THE FOLLOWING ESTIMATED QUANTITIES ARE TO BE USED AS DIRECTED BY THE ENGINEER FOR EROSION AND SILTATION CONTROL MEASURES:

ITEM 207 STRAW OR HAY BALES	120 EACH
-----------------------------	----------

DITCH CHECKS AS SHOWN ON STANDARD DRAWING MC-11 SHALL BE PROVIDED IN EVERY DITCH THAT IS NOT OTHERWISE PROTECTED AGAINST EROSION. THE DITCH CHECKS SHALL BE LOCATED NEAR THE POINT WHERE THE DITCH DISCHARGES INTO THE STREAM CHANNEL. CATCH BASINS SHALL BE PROTECTED WITH BALE INLET FILTERS. BALE FILTER DIKES SHALL BE PLACED ALONG DITCHES LINED WITH SOD OR JUTE MATTING, AND ALONG TOE OF EMBANKMENTS WITHOUT DITCHES.

STRAW OR HAY BALES SHALL REMAIN IN PLACE UNTIL SEEDING IS WELL ESTABLISHED OR UNTIL ORDERED REMOVED BY THE ENGINEER.

DETOUR LIMITATION

TWO-WAY TRAFFIC SHALL BE MAINTAINED AT ALL TIMES, EXCEPT THAT FOR A PERIOD NOT TO EXCEED 120 CALENDAR DAYS, THROUGH TRAFFIC MAY BE DETOURED AS SHOWN ON SHEET 2.

THE CONTRACTOR SHALL NOTIFY THE DISTRICT TRAFFIC ENGINEER IN WRITING AT LEAST TEN CALENDAR DAYS BEFORE THE PROPOSED START OF THE DETOUR.

THE CONTRACTOR SHALL BE ASSESSED \$100 PER DAY IN LIQUIDATED DAMAGES FOR EACH ADDITIONAL CALENDAR DAY THE DETOUR REMAINS IN EFFECT BEYOND THE ABOVE STATED DETOUR PERIOD, OR SUCH OTHER DETOUR PERIOD WHICH HAS BEEN AGREED UPON DUE TO CONDITIONS BEYOND THE CONTRACTOR'S CONTROL. *SIGNS, LIGHTS AND BARRICADES NEEDED FOR DETOUR WILL BE PROVIDED AND MAINTAINED BY THE STATE OF OHIO.*

MAINTAINING TRAFFIC

LOCAL ACCESS TO DRIVES WITHIN THE PROJECT LIMITS SHALL BE MAINTAINED AT ALL TIMES. THE FOLLOWING ESTIMATED QUANTITIES ARE PROVIDED TO MAINTAIN LOCAL TRAFFIC AS DIRECTED BY THE ENGINEER.

ITEM 410	TRAFFIC COMPACTED SURFACE, TYPE A OR B	50 CU. YD
ITEM 616	CALCIUM CHLORIDE	0.5 TON

614 TEMPORARY PAVEMENT MARKINGS

THE CONTRACTOR SHALL FURNISH AND INSTALL TEMPORARY RETROREFLECTIVE PAVEMENT MARKINGS WITHIN THE WORK LIMITS, IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS. CENTER LINES SHALL CONSIST OF 12" x 4" SEGMENTS SPACED AT A MAXIMUM 40' CENTER TO CENTER.

THE MATERIAL FURNISHED SHALL BE FLEXIBLE RETROREFLECTIVE PREFORMED PRESSURE SENSITIVE TAPE FOR PAVEMENT LINES. IT SHALL BE FREE OF CRACKS WITH STRAIGHT EDGES AND CONSIST OF PIGMENT AND FILLERS, BUT HAVE SUFFICIENT BINDER AND PLASTICIZER TO RETAIN GLASS BEADS HAVING AN APPROPRIATE REFRACTIVE INDEX TO MEET MINIMUM REFLECTIVE INTENSITY STANDARDS OUTLINED IN THE MANUFACTURERS INFORMATION. MATERIALS SHALL BE FLEXOLITE "WET REFLECTIVE," 3 M "SCOTCH-LANE," OR APPROVED EQUAL.

GLASS BEADS SHALL BE MIXED UNIFORMLY THROUGHOUT THE MARKING MATERIAL WITH SUFFICIENT SURFACE BEADS TO PROVIDE OPTIMUM REFLECTORIZATION AT ALL TIMES. THE MATERIAL SHALL HAVE A PRECOATED ADHESIVE LAYER FOR PAVEMENT APPLICATION WITHOUT THE USE OF HEAT, SOLVENTS, OR ADDITIONAL ADHESIVES. THE ADHESIVE SHALL BE SUFFICIENT TO RETAIN COMPLETE MARKINGS ON THE PAVEMENT SURFACE THROUGHOUT THE USEFUL LIFE OF THE MARKINGS. YELLOW MATERIAL SHALL CONFORM TO COLOR NO. 35538 OF FEDERAL STANDARD 595.

IN ADDITION, ALL APPLICABLE MANUFACTURERS MATERIAL AND APPLICATION INSTRUCTIONS, IN FORCE AT THE TIME OF PLACEMENT, SHALL BE ADHERED TO. THE CONTRACTOR SHALL FURNISH TO THE ENGINEER CERTIFICATION THAT THE MATERIAL SUPPLIED MEETS THE PROPERTIES SPECIFIED HERE. MARKINGS SHALL BE ACCURATELY LAID OUT IN CONFORMANCE WITH 621.051 AND SHALL BE LOCATED IN A TRUE LINE ON THE CENTER LINE. THE TEMPORARY TAPE SHALL BE PLACED BY ROLLING THE MATERIAL INTO THE SURFACE.

"AS AN ALTERNATE MATERIAL TO PAVEMENT MARKING TAPE, THE CONTRACTOR MAY FURNISH AND APPLY PAINTED RETROREFLECTIVE PAVEMENT MARKINGS CONFORMING TO 621. THE WIDTH AND LENGTH OF PAINTED SEGMENT SHALL BE THE SAME AS REQUIRED FOR TEMPORARY TAPE MATERIAL. THE PAINT APPLICATION RATE SHALL BE NOT LESS THAN 16 GALLONS PER MILE FOR A SOLID LINE NOR LESS THAN 0.4 GALLONS PER MILE FOR THE 12" x 4" DASHED LINE."

METHOD OF MEASUREMENT AND BASIS OF PAYMENT SHALL BE IN CONFORMANCE WITH 621.15 AND 621.16 RESPECTIVELY FOR:

ITEM	UNIT	DESCRIPTION
614	0.10 MILES	TEMPORARY CENTER LINES

PERMANENT PAVEMENT MARKINGS

THE STATE OF OHIO SHALL FURNISH AND PLACE PERMANENT PAVEMENT MARKINGS WITHIN 30 DAYS FOLLOWING COMPLETION OF ALL SURFACE COURSES.

FARM DRAINS (SEE PLAN AND PROFILE SHEET NO. 5)

MOBILIZATION, AS PER PLAN

THE CONTRACTOR SHALL PROVIDE SUITABLE FIELD OFFICE, HAVING A MINIMUM OF 150 SQ. FT. OF FLOOR SPACE WHICH SHALL BE IN ACCORDANCE WITH G19.01 AND G19.02. PAYMENT SHALL BE INCLUDED IN THE LUMP-SUM PRICE BID FOR ITEM G24, MOBILIZATION, AS PER PLAN.

ROUNDING OF CORNERS AS SHOWN ON CROSS SECTIONS

THE ROUNDED CORNERS SHOWN ON THE TYPICAL SECTIONS APPLY TO ALL CROSS SECTIONS EVEN THOUGH OTHERWISE SHOWN ON THESE PLANS.

ELEVATION DATUM

ALL ELEVATIONS ARE BASED ON THE USGS DATUM.

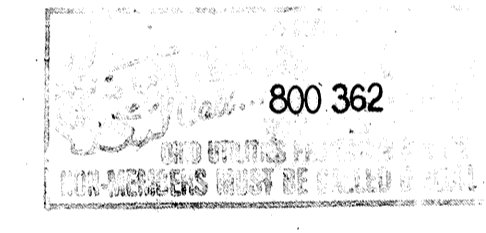
UNDERGROUND UTILITIES

THE LOCATIONS OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS HAVE BEEN OBTAINED BY DILIGENT FIELD CHECKS AND SEARCHES OF THE AVAILABLE RECORDS. IT IS BELIEVED THAT THEY ARE ESSENTIALLY CORRECT, BUT THE STATE OF OHIO DOES NOT GUARANTEE THEIR ACCURACY OR COMPLETENESS.

UTILITY OWNERSHIP

THE FOLLOWING UTILITIES AND OWNERS ARE LOCATED WITHIN THE WORK LIMITS OF THIS PROJECT:

POWER	OHIO EDISON CO. 76 SOUTH MAIN ST. AKRON, OHIO 44308 (216) 384-5234
TELEPHONE	GENERAL TELEPHONE COMPANY OF OHIO 108 W. WASHINGTON MEDINA, OHIO 44256 (216) 725-8411
GAS	COLUMBIA GAS TRANSMISSION CORP. P.O. BOX 1273 CHARLESTON, W. VA. (304) 346-0951



CONTINGENCY QUANTITIES

SPECIFIC LOCATIONS AND USAGE OF ESTIMATED QUANTITIES SET UP ON THIS PLAN TO BE USED "AS DIRECTED BY THE ENGINEER" SHALL BE MADE A MATTER OF RECORD BY INCORPORATION INTO THE FINAL CHANGE ORDER GOVERNING COMPLETION OF THIS PROJECT. ESTIMATED QUANTITIES OF MATERIALS SHALL NOT BE ORDERED FOR DELIVERY TO THE PROJECT UNLESS AUTHORIZED BY THE ENGINEER.

APPROACH SLABS

JACKING HOLES, AS SHOWN ON AS-1-72, SHALL BE OMITTED & THE CONCRETE COVER OVER THE TOP REINFORCING STEEL SHALL BE INCREASED FROM 2 INCHES TO 3 INCHES. THE TOP CONCRETE SURFACE OF THE APPROACH SLABS SHALL VARY IN DISTANCE BELOW THE FINISHED GRADE FROM 2 1/2 INCHES AT STATIONS 498+94.33 AND 499+85.67 TO 4 INCHES AT STATIONS 499+19.33 AND 499+60.67, RESPECTIVELY.

LOCATION OF GUARDRAIL

THE LOCATIONS OF GUARDRAIL RUNS, AS SHOWN IN THESE PLANS, ARE SUBJECT TO ADJUSTMENT TO ASSURE THAT THE PLANNED INSTALLATIONS WILL AFFORD MAXIMUM PROTECTION FOR TRAFFIC.

COVER AGGREGATE FOR TACK COAT

The aggregate for item 407 Cover Aggregate shall conform to 703.06.

CALCULATIONS

LINE	DESCRIPTION	QUANTITIES
1	404 1" ASPHALT CONCRETE	
2		
3	From Typical Sections: 158.66 Lin. Ft.	
4	From Line 3: 158.66' x 24.00' = 3807.84 Sq. Ft.	
5	Add Taper: Sta. 497+50 To Sta. 498+50 = 100.00 Lin. Ft.	
6	Add Taper: Sta. 501+00 To Sta. 502+00 = 100.00 Lin. Ft.	
7	From Line 5 & 6: (100'+100')(22' Average Width) = 4400 Sq. Ft.	
8	Add Feather: Sta. 497+00 To Sta. 497+50 = 50.00 Lin. Ft.	
9	Add Feather: Sta. 502+00 To Sta. 502+50 = 50.00 Lin. Ft.	
10	Add Line 8 & 9: (50'+50')(20') = 2000 Sq. Ft.	
11	Add Approach Slab: Sta. 498+94.33 To Sta. 499+19.33 = 25.00 Lin. Ft.	
12	Add Approach Slab: Sta. 499+60.67 To Sta. 499+85.67 = 25.00 Lin. Ft.	
13	From Line 11 & 12: $(\sqrt{25^2+44^2})(25' \times \sin 60^\circ)(2) = 2191.32$ Sq. Ft.	
14	From Line 4, 7, 10 & 13: $(3807.84+4400+2000+2191.32)(\frac{1}{2}) \div 27 = 38.25$ Cu. Yds.	Use 38 Cu. Yds.
15		
16	404 VARIABLE ASPHALT CONCRETE	
17		
18	From Line 8 & 9: $(50'+50')(0.13')(20)(\frac{1}{2}) \div 27 = 4.82$ Cu. Yds.	
19	Add Approach Slabs: $[(\sqrt{25^2+44^2})(25' \times \sin 60^\circ)(0.13') + \frac{1}{2}(0.13')(1095.66)] \div 27 \times 2 = 15.84$ Cu. Yds.	
20	From Line 18 & 19: 4.82+15.84 = 20.66 Cu. Yds.	
21		Use 21 Cu. Yds.
22	402 1 1/2" ASPHALT CONCRETE	
23		
24	From Line 4 & 7: $(3807.84 + 4400.00)(\frac{1 1/2"}{2}) \div 27 = 38.00$ Cu. Yds.	Use 38 Cu. Yds.
25		
26	301 6" BITUMINOUS AGGREGATE BASE	
27		
28	From Line 3: $(158.66')(25' \text{ Average Width}) = 3966.50$ Sq. Ft.	
29	From Line 5 & 6: $(100'+100')(23' \text{ Average Width}) = 4600.00$ Sq. Ft.	
30	From Line 28 & 29: $(3966.50 + 4600.00)(\frac{6"}{12}) \div 27 = 159.64$ Cu. Yds.	Use 159 Cu. Yds.
31		
32	203 SUBGRADE COMPACTION	
33		
34	From Line 4, 7 & 13: $(3807.84 + 4400 + 2191.32) \div 9 = 1155.46$ Sq. Yds.	Use 1156 Sq. Yds.
35		
36	611 REINFORCED CONCRETE APPROACH SLAB	
37		
38	From Line 13: $(2191.32) \div 9 = 243.48$ Sq. Yds.	Use 244 Sq. Yds.
39		
40	407 TACK COAT	
41		
42	From Line 8 & 9: $(50'+50')(20') \div 9 \times 0.10 \text{ Gal./Per Sq. Yd.} = 22.22$ Gals.	Use 22 Gals.
43		
44	407 COVER AGGREGATE	
45		
46	From Line 8 & 9: $(50'+50')(20') \div 9 \times 7 \text{ Lbs./Sq. Yd.} \div 2000 \text{ Lbs./Ton} = 0.78$ Tons	Use 0.78 Tons
47		
48	304 8" AGGREGATE BASE	
49		
50	From Line 3, 5, 6, 8 & 9: $(158.66'+100'+100'+50'+50')(2) = 917.32$ Lin. Ft.	
51	From Line 50: $(917.32)(3.538 \text{ Sq. Ft./Ft.}) \div 27 = 120.20$ Cu. Yds.	
52	Minus Area Through Drives: $120.20 - 3.86 = 116.34$ Cu. Yds.	Use 116 Cu. Yds.
53		
54	408 BITUMINOUS PRIME COAT	
55		
56	From Line 50: $(917.32)(\frac{1}{4}) \div 9 = 407.70$ Sq. Yds.	
57	Minus Areas Through Drives: $407.70 - 42.89 = 364.81$ Sq. Yds.	
58	From Line 57: $(364.81)(0.4 \text{ Gal./Sq. Yd.}) = 145.92$ Gals.	Use 146 Gals.
59		
60	409 SEAL COAT BITUMINOUS PRIME COAT	
61		
62	From Line 57: $(364.81)(0.30 \text{ Gal./Sq. Yd.}) = 109.44$ Gals.	Use 110 Gals.
63		
64	409 SEAL COAT COVER AGGREGATE NO. 8	
65		
66	From Line 57: $(364.81)(0.008 \text{ Cu. Yd./Sq. Yd.}) = 2.92$ Cu. Yds.	Use 3 Cu. Yds.
67		
68	659 COMMERCIAL FERTILIZER	
69		
70	From Summary: 659 Seeding = 3400 Sq. Yds.	
71	From Line 70: $(3400)(9) \div 1000 \times (20 \div 2000) = 0.31$ Tons	Use 0.31 Tons
72		

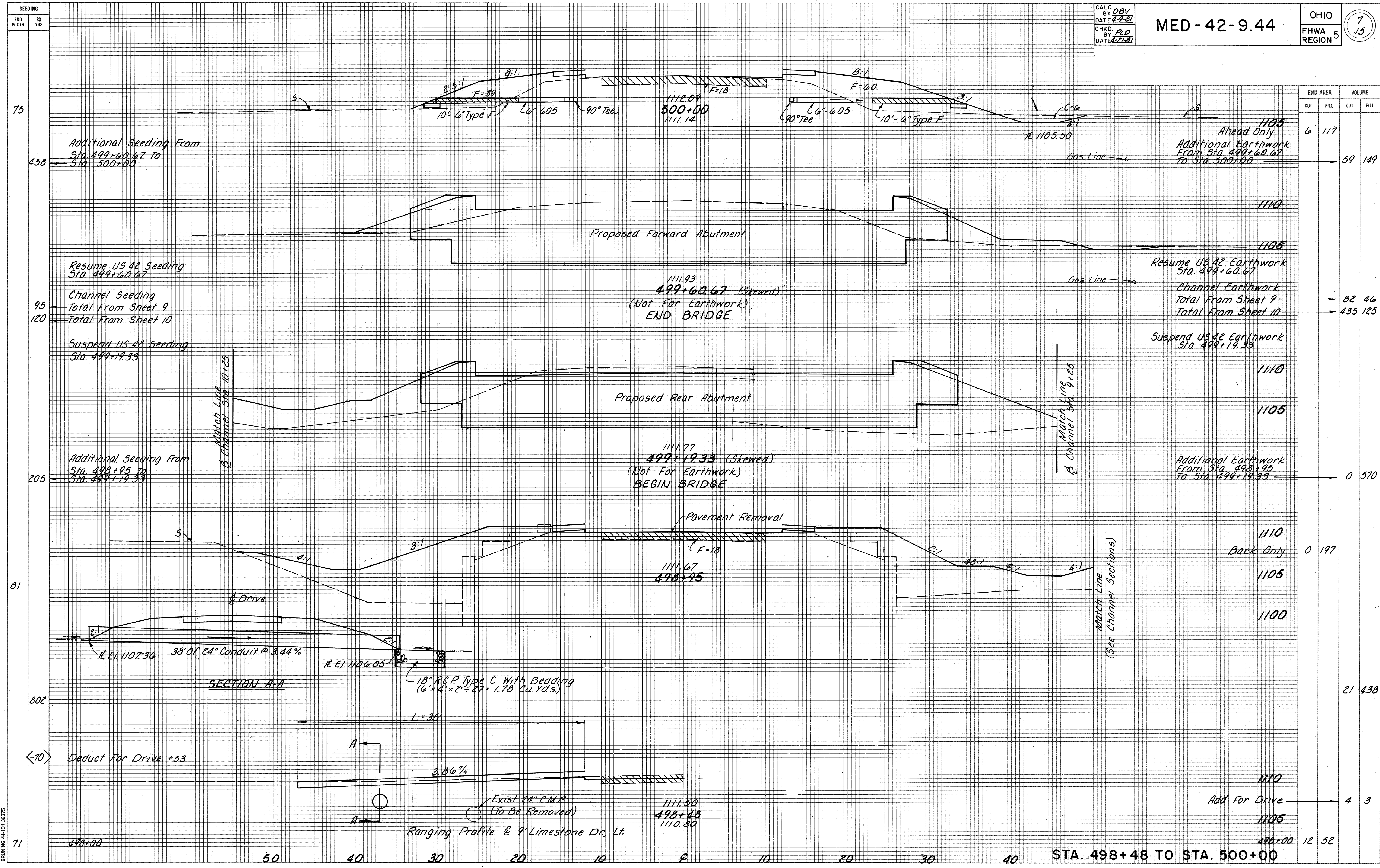
GENERAL SUMMARY

General Notes	FROM SHEET NUMBER			ITEM NO.	TOTAL QUANTITIES	UNIT	DESCRIPTION
	4	5					
							ROADWAY
Lump				201	Lump		Clearing And Grubbing
		956		202	956	Sq. Yd.	Pavement Removed
		103		202	103	Lin. Ft.	Pipe Removed 24" And Under
		406		202	406	Lin. Ft.	Guardrail Removed
		702		203	702	Cu. Yd.	Excavation Not Including Embankment Construction
		1967		203	1967	Cu. Yd.	Embankment
50	1156			203	1156	Sq. Yd.	Subgrade Compaction
				410	50	Cu. Yd.	Traffic Compacted Surface, Type A or B
		392.32		606	392.32	Lin. Ft.	Guard Rail, Type 5
		3		606	3	Each	Anchor Assembly, Standard Type A
		1		606	1	Each	Anchor Assembly, Standard Type T
		4		606	4	Each	Bridge Terminal Assembly, Standard Type B
50				616	50	M Gal.	Water
1				616	1	Ton	Calcium Chloride
0.10				614	0.10	Mile	Temporary Center Lines
							EROSION CONTROL
120				207	120	Each	Hay or Straw Bales
		2		601	2	Cu. Yd.	Rock Channel Protection, Type C, With Bedding
		239		601	239	Cu. Yd.	Rock Channel Protection, Type C, Without Bedding
		2		601	2	Cu. Yd.	Rock Channel Protection, Type D, With Bedding
		3400		659	3400	Sq. Yd.	Seeding and Mulching
4	0.31			659	0.31	Tons	Commercial Fertilizer
				659	4	M Gal.	Water
		322		667	322	Sq. Yd.	Seeding and Jute Matting
							DRAINAGE
		82		603	82	Lin. Ft.	12" Conduit, Type D
		36		603	36	Lin. Ft.	15" Conduit, Type D
		38		603	38	Lin. Ft.	24" Conduit, Type D
		12		603	12	Lin. Ft.	4" Conduit, Type E, 711.29
		80		603	80	Lin. Ft.	6" Conduit, Type F
		1		604	1	Each	Catch Basin, Standard No. 2-2-B
		334		605	334	Lin. Ft.	6" Shallow Pipe Underdrains
		142		605	142	Lin. Ft.	6" Unclassified Pipe Underdrains
							PAVEMENT
		159	30	301	189	Cu. Yd.	Bituminous Aggregate Base: AC-20, RT-11, or RT-12
		116	13	304	129	Cu. Yd.	Aggregate Base
		38		402	38	Cu. Yd.	Asphalt Concrete, AC-20
		59		404	59	Cu. Yd.	Asphalt Concrete, AC-20
			11	404	11	Cu. Yd.	Asphalt Concrete, AC-20 (For Driveways)
		22		407	22	Gals	Tack Coat
		0.78		407	0.78	Tons	Cover Aggregate
		146		408	146	Gals.	Bituminous Prime Coat
		110		409	110	Gals.	Seal Coat Bituminous Material
		3		409	3	Cu. Yd.	Seal Coat Cover Aggregate, No. 8
		244		611	244	Sq. Yd.	Reinforced Concrete Approach Slab (T=15")
							STRUCTURES
							STRUCTURE MED-42-0946 (Quantities on Sheet 12)
Lump				614	Lump		Maintaining Traffic
Lump				623	Lump		Construction Layout Stakes
Lump				624	Lump		Mobilization, As Per Plan

CALC. BY
DATE 2-2-81
CHKD. BY
DATE 2-2-81

MED-42-9.44

OHIO
FHWA REGION 5
7
15



END AREA	VOLUME	
	CUT	FILL
1105	6	117
1110		59
1105		149
1110		
1105		82
1110		46
1105		435
1110		125
1105		
1110		0
1105		570
1110		
1105	0	197
1110		
1105		21
1110		438
1105		
1110		4
1105		3
1110		
1105	12	52

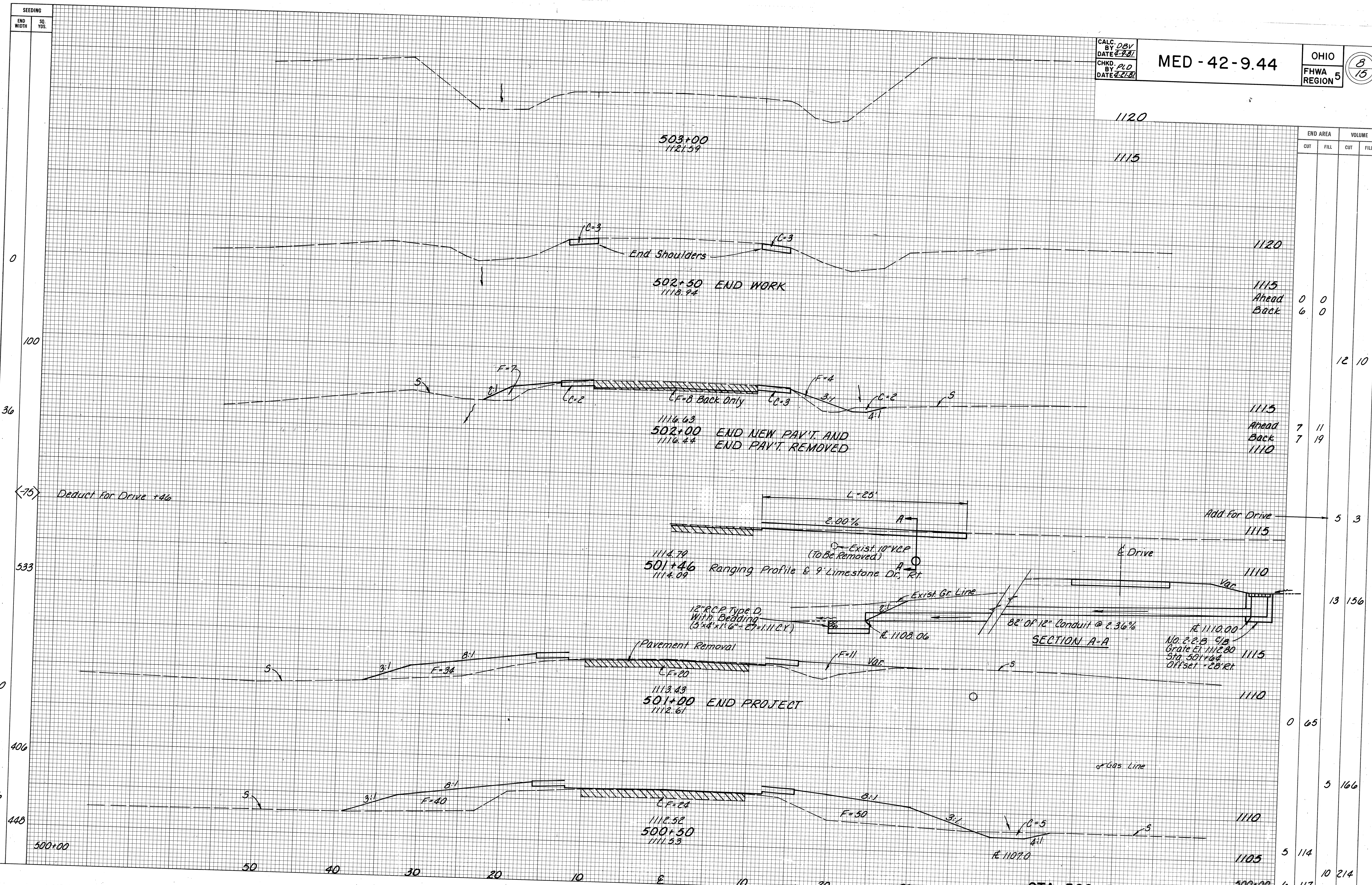
BRUNING 44-131 38375

CALC. BY: DBV
 DATE: 4-28-01
 CHKD. BY: PLO
 DATE: 4-21-01

MED - 42-9.44

OHIO
 FHWA REGION 5

8/15



END AREA	VOLUME	
	CUT	FILL
1120	0	0
1115 Ahead	6	0
1115 Back	0	0
1115	7	11
1110 Ahead	7	19
1110 Back	0	0
1115	5	3
1110	13	156
1110	0	65
1110	5	166
1105	5	114
1110	10	214
500+00	6	117

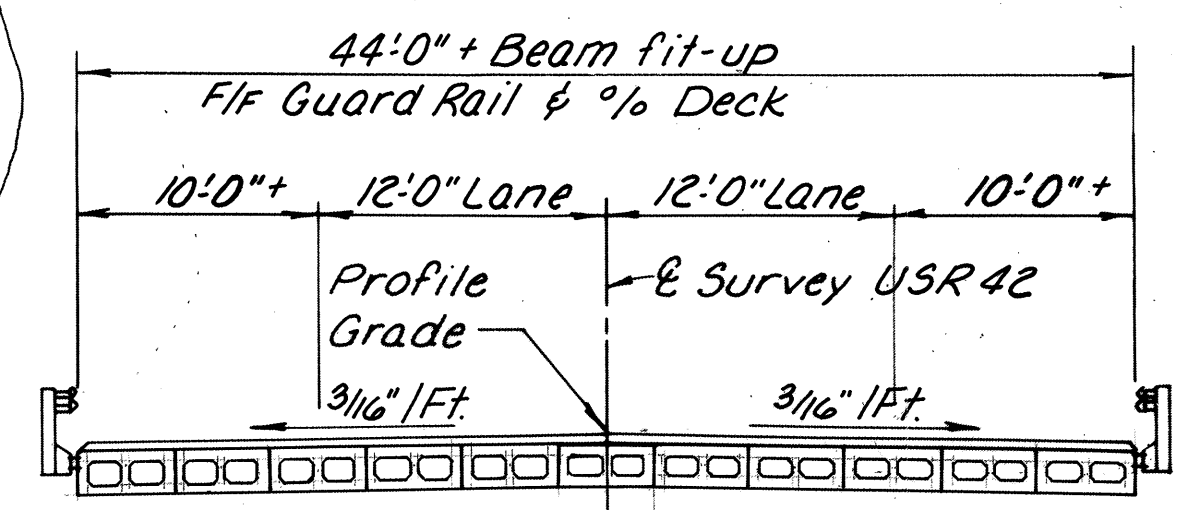
BRUNING 44-131 38375

BM Boat spike in # 217611 P-3
Sta. 499+90.5, 25.5' Rt.
Elev. 1108.35

MICROFILMED
FEB 6 1987

OHIO	11
FHWA REGION 5	15

CALC. BY _____
DATE _____
CHKD BY _____
DATE _____



TRANSVERSE SECTION

EXISTING STRUCTURE

BRIDGE NO. MED-42-0945
TYPE: The Superstructure consists of a combination of the original deck built in 1918 of cast-in-place reinforced concrete T-beams and reinforced concrete slab extensions added on each side in 1931 and 1944. Substructure units are gravity abutments made of concrete and stone.
SPAN: 18'-0" F/F abutments
ROADWAY: 30'-0" F/F guardrail
LOADING: H-15 (S-11.7-46)
SKEW: 0°
WEARING SURFACE: Bituminous
APPROACH SLABS: None
ALIGNMENT: Tangent
GENERAL CONDITION: Poor, To be removed.

PROPOSED STRUCTURE

TYPE: Single span prestressed concrete box beams on reinforced concrete abutments
SPAN: 40'-0" C/C Bearings
ROADWAY: 44'-0" F/F guardrail
LOADING: HS-20-44 and alternate military loading.
SKEW: 30° 00' L.F.
WEARING SURFACE: 2 1/2" Asphalt concrete
APPROACH SLABS: A5-1-72 (25' long)
ALIGNMENT: Tangent
AVERAGE DAILY TRAFFIC: 4200 (Yr. 2000)

FOUNDATION INVESTIGATION LEGEND

● Indicates Core Boring Location
EARTHWORK limits shown are approximate. Actual slopes shall conform to plan cross-sections.

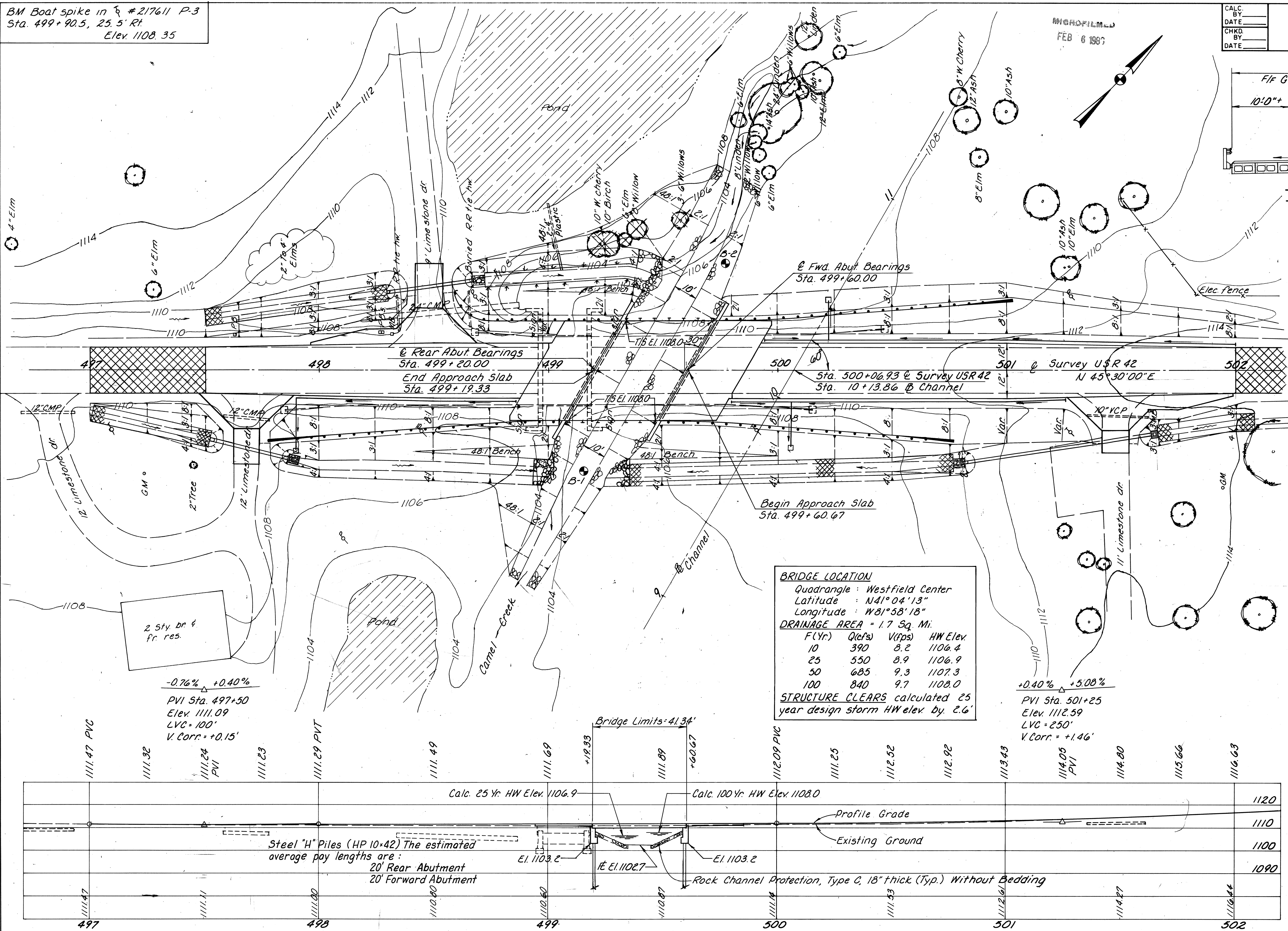
ENGINEERING ASSOCIATES LTD.
CONSULTING ENGINEERS
700 WINKLER DR. WOOSTER, OHIO

SITE PLAN

BRIDGE NO. MED-42-0945
OVER CAMEL CREEK

MEDINA COUNTY U.S. RT. 42
STA. 499+19.33 TO STA. 499+60.67

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
DBC	DBV	PLD	DWS	WRH	9/21/81	



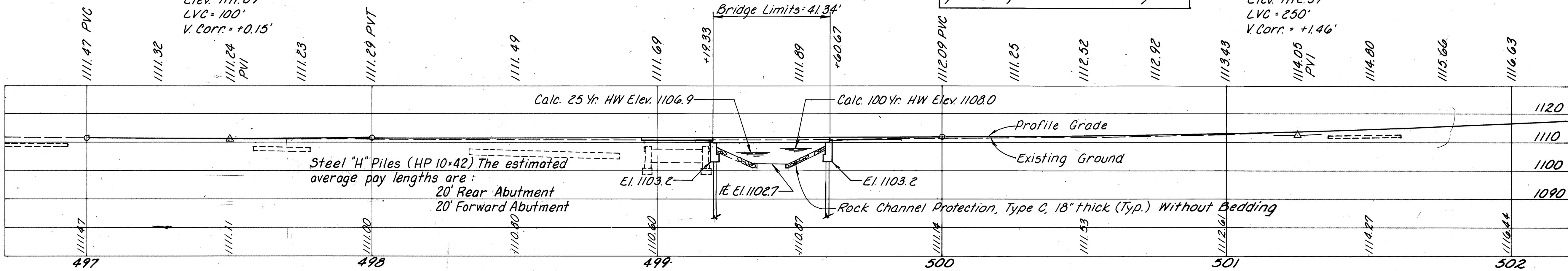
BRIDGE LOCATION

Quadrangle: Westfield Center
Latitude: N41° 04' 13"
Longitude: W81° 58' 18"

DRAINAGE AREA = 1.7 Sq. Mi.

F (Yr.)	Q (cfs)	V (fps)	HW Elev.
10	390	8.2	1106.4
25	550	8.9	1106.9
50	685	9.3	1107.3
100	840	9.7	1108.0

STRUCTURE CLEARS calculated 25 year design storm HW elev. by 2.6'



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GENERAL NOTES

REFERENCE shall be made to Standard Drawings:

- AS-1-72 sheet 1 of 2 dated 6-30-72
- DBR-2-73 dated 4-10-73
- PSBD-1-81, sheets 1, 2 and 3 of 4 dated 9-18-81.

DESIGN SPECIFICATIONS: This structure conforms to the "Standard Specifications for Highway Bridges" adopted by the American Association of State Highway and Transportation Officials, 1977, including the 1978 through 1980 Interim Specifications and the Ohio "Supplement" to these Specifications.

DESIGN DATA:

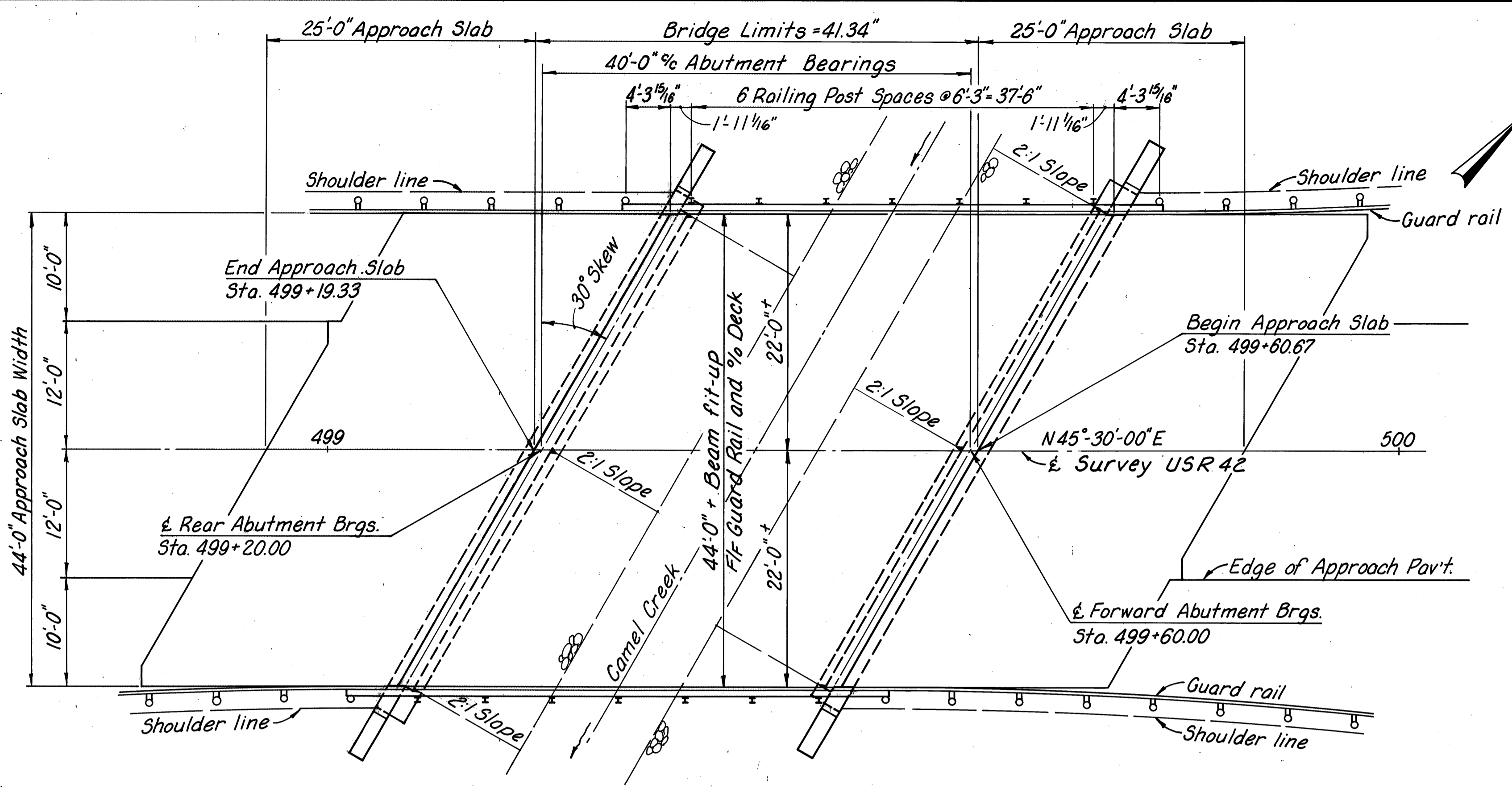
- Design loading - HS20-44 and the Alternate Military Loading
- Concrete Class C - Unit stress 1333 psi for substructure
- Reinforcing Steel - ASTM A615, A616 or A617 - Unit stress 20,000 psi.
 Splice lengths indicated in the plans are for Grade 60 steel and $f_c' = 4000$ p.s.i.
- Concrete for prestressed concrete beams - Unit stress 2200 p.s.i. compression
 444 p.s.i. tension
- Prestressing strand ASTM A416 - $f_s = 270,000$ p.s.i.
 Initial stress 0.70 f_s
- Deck Protection Method: Primary - Membrane waterproofing and asphalt concrete overlay with steel drip strip along edges of deck.
 Supplementary - None Required

REMOVAL OF EXISTING STRUCTURE: When no longer needed to maintain traffic the existing structure shall be removed in accordance with CMS Section 202.03. Suitable waste masonry may be placed as Bank Protection as directed by the Engineer.

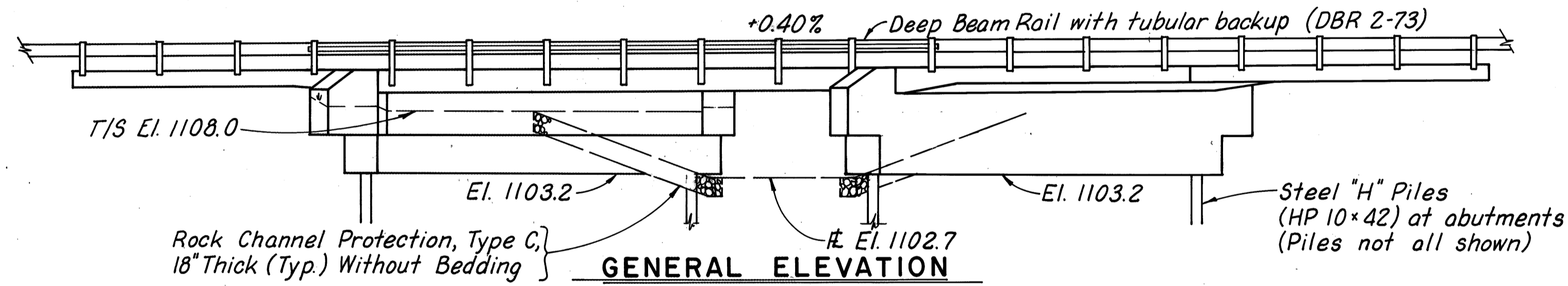
EMBANKMENT CONSTRUCTION: The embankments shall be constructed to the level of the subgrade for a minimum distance of 200 feet back of the abutments. Excavation may then be made for the abutments and piles driven.

PILES shall be driven to a minimum bearing capacity of 38 tons per pile for the abutments.

ADDITIONAL GENERAL NOTES: See Referenced Standard Drawings and Project General Notes Sheet 3 of 15 for approach slab notes regarding concrete cover over top reinforcement, jacking holes, and longitudinal grade of approach slab.



GENERAL PLAN



GENERAL ELEVATION

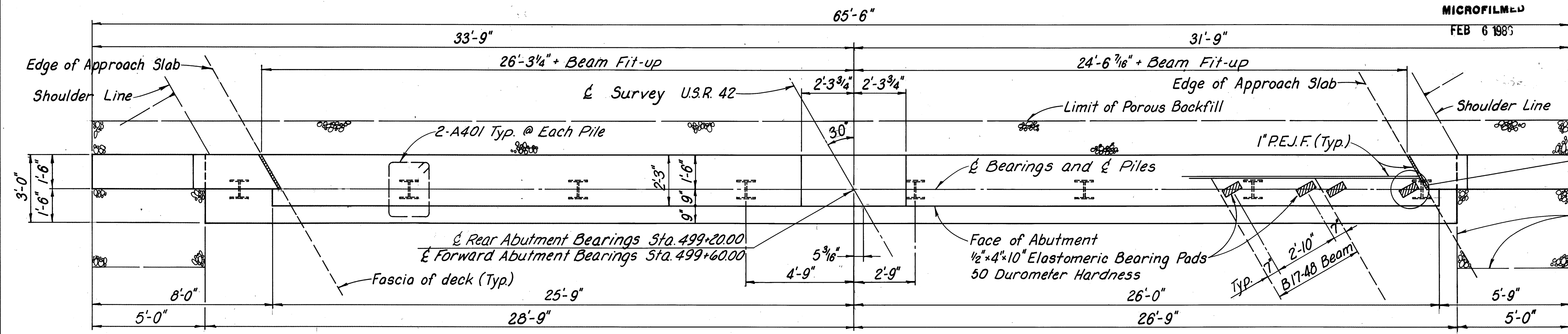
ESTIMATED QUANTITIES						
ITEM	TOTAL	UNIT	DESCRIPTION	SUPER	ABUTS.	GEN'L.
202	Lump	Sum	Structure removed			Lump
403	11	Cu.Yd.	Asphalt concrete AC 20	11		
404	6	Cu.Yd.	Asphalt concrete AC 20	6		
503	116	Cu.Yd.	Unclassified excavation		116	
505	Lump	Sum	Test Pile			Lump
507	320	Lin.Ft.	Steel piles HP 10x42		320	
509	7593	Lb.	Reinforcing steel		7593	
510	12	Each	Dowel holes		12	
511	76	Cu.Yd.	Class C concrete, abutments		76	
512	224	Sq.Yd.	Type D Waterproofing	224		
515	11	Each	Prestressed concrete bridge members, B17-48	11		
516	44	Each	1/2x4x10 Elastomeric bearing pads	44		
516	6	Sq.Ft.	1/8x4x10 Preformed bearing pads, 711.21	6		
516	145	Sq.Ft.	1" Preformed expansion joint filler		145	
516	102	Lin.Ft.	Joint Sealer		102	
517	82.68	Lin.Ft.	Railing (Deep beam with tubular backup, Type 2 steel posts and bolts)	82.68		
518	33	Cu.Yd.	Porous backfill		33	
Special	63	Sq.Ft.	Steel drip strip	63		

2/4

800

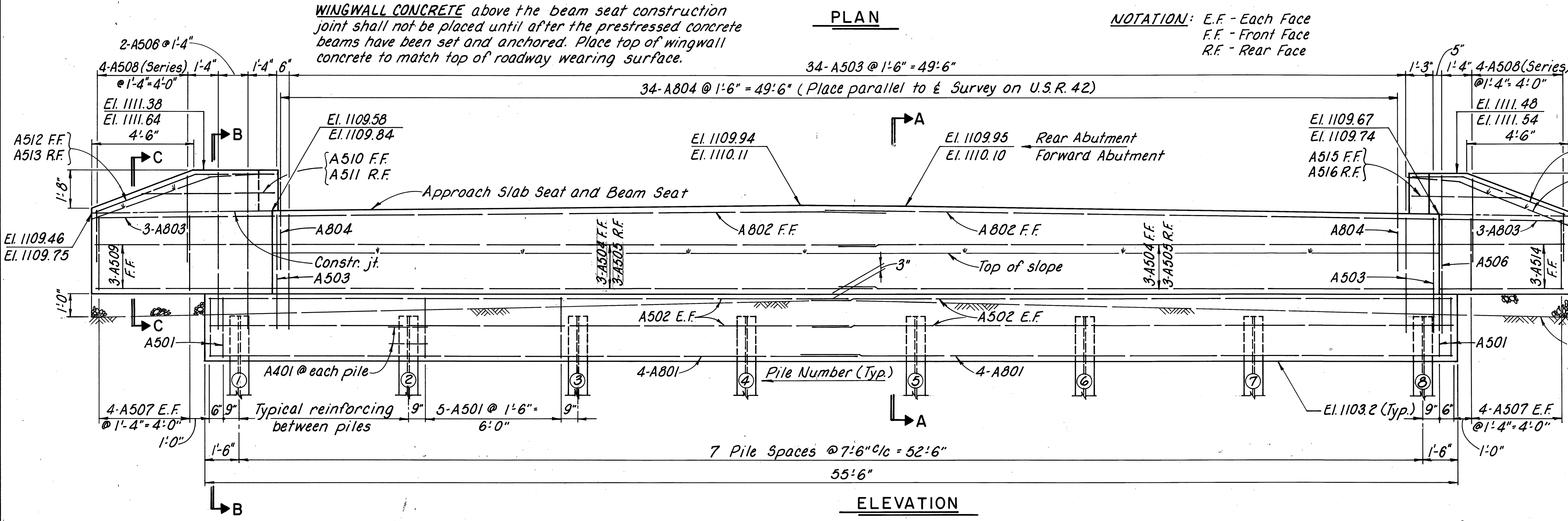
FLYNN WARDWELL

ENGINEERING ASSOCIATES LTD.					
CONSULTING ENGINEERS 700 WINKLER DR. WOOSTER, OHIO					
GENERAL PLAN, GENERAL NOTES AND ESTIMATED QUANTITIES					
BRIDGE NO. MED-42-0945 OVER CAMEL CREEK					
MEDINA COUNTY US. RT. 42 STA. 499+19.33 TO STA. 499+60.67					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE
DBC	RLE	RLE	DWS	WRH	9/21/81

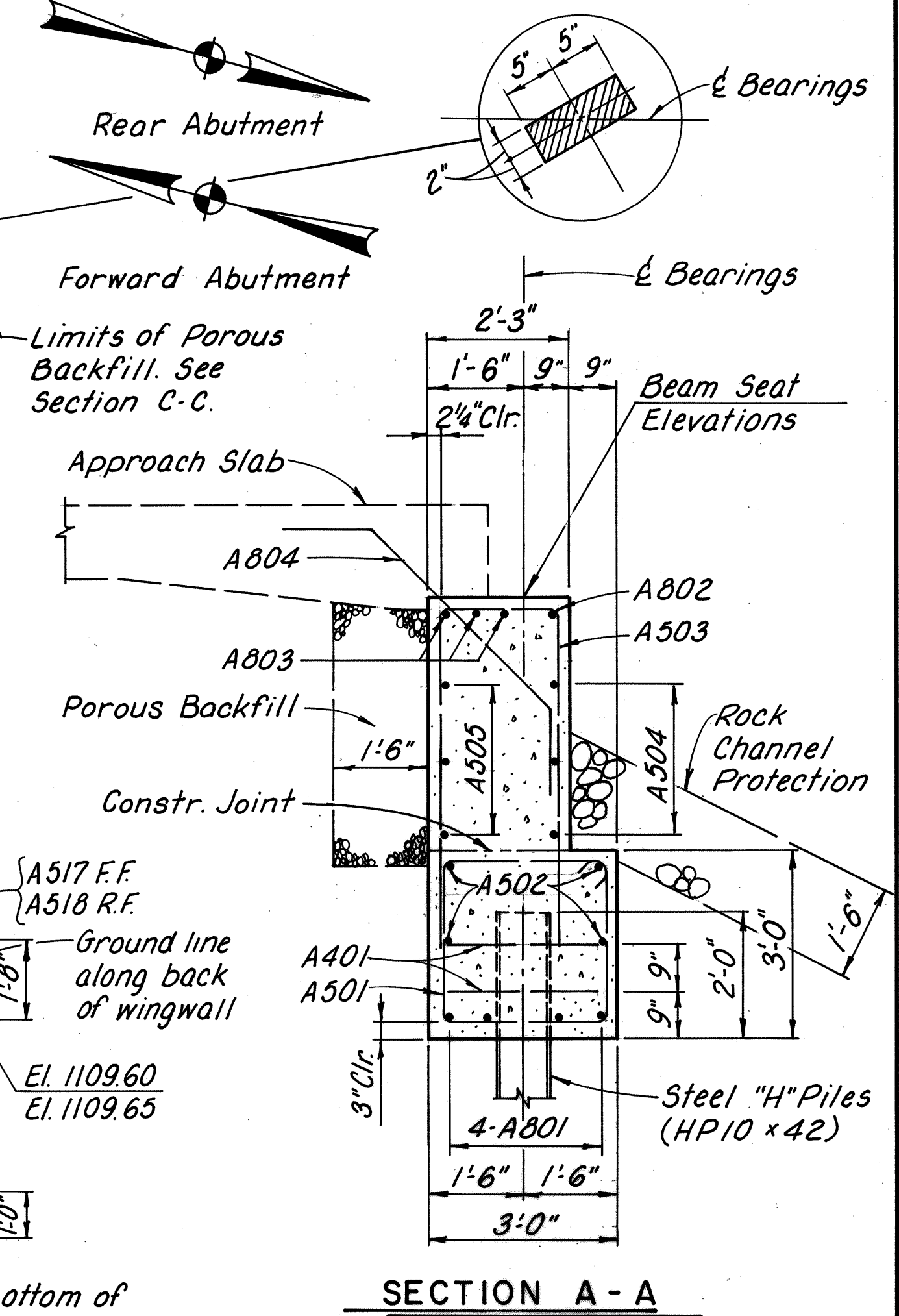


PLAN

NOTATION: E.F. - Each Face
F.F. - Front Face
R.F. - Rear Face



ELEVATION



SECTION A-A

ABUTMENT NOTES

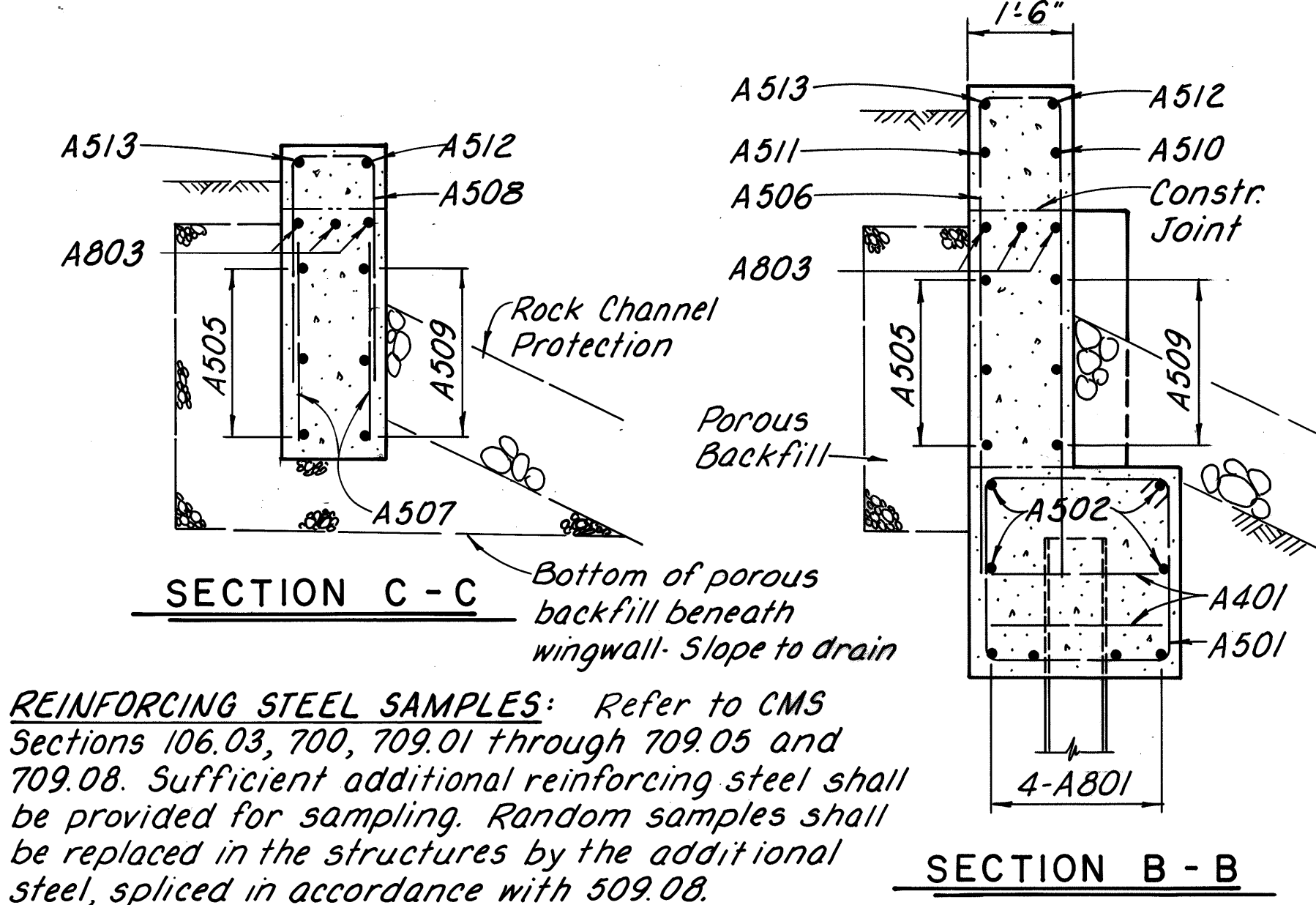
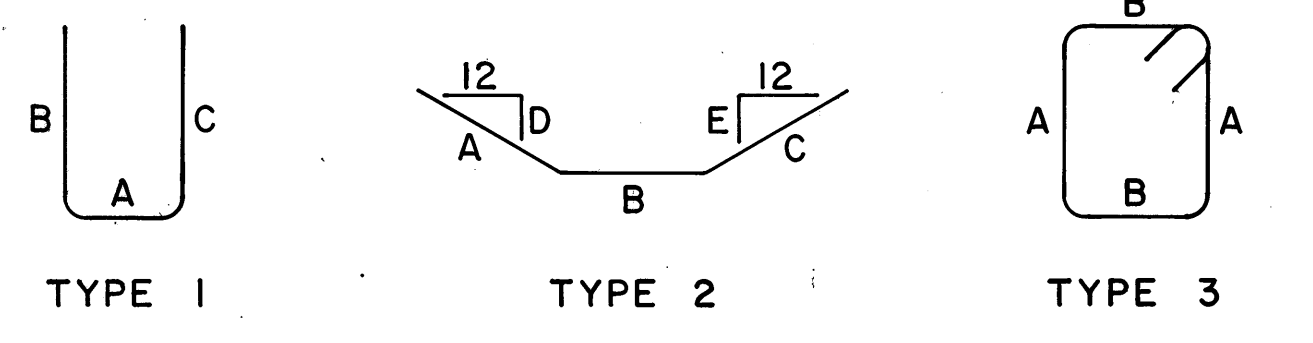
POROUS BACKFILL shall extend upward to the plane of the subgrade, laterally to the ends of the wingwalls, and beneath the wingwalls to the Rock Channel Protection.
BEAM SEAT REINFORCING: Reinforcing steel in the vicinity of the beam seat shall be accurately placed to avoid interference with the drilling of anchor bolt holes.
HORIZONTAL WINGWALL REINFORCING above the bridge seat construction joint shall be field shortened as required by beam fit-up.

REINFORCING SPLICE LENGTHS shall be 1'-7" (min.) for # 5 bars
2'-7" (min.) for # 8 bars

GENERAL NOTES: See Sheet 2/4

ABUTMENTS																				
MARK	NO.	LENGTH	TYPE	A	B	C	D	E	WEIGHT	MARK	NO.	LENGTH	TYPE	A	B	C	D	E	WEIGHT	
A401	32	8'-10"	3	2'-5"	1'-9"				189	A517	2	6'-2"	2	1'-6"	4'-8"	0	4 1/2	0	13	
										A518	2	6'-9"	2	2'-1"	4'-8"	0	4 1/2	0	14	
A501	78	11'-1"	3	2'-7"	2'-8"				902	A601	12	2'-4"	Str.						42	
A502	16	28'-5"	Str.						455	A801	16	28'-11"	Str.						1235	
A503	72	12'-9"	1	1'-11"	5'-6"	5'-6"			957	A802	4	27'-0"	Str.						288	
A504	12	26'-6"	Str.						332	A803	12	33'-11"	Str.						1087	
A505	12	33'-5"	Str.						418	A804	68	6'-6"	2	1'-5"	3'-10"	1'-5"	12	12	1180	
A506	6	15'-0"	1	1'-2"	7'-0"	7'-0"			94											
A507	32	2'-10"	Str.						95											
A508	16	5'-6" to 8'-6"	1	1'-2"	*	*			117											
A509	6	9'-3"	Str.						58											
A510	2	5'-9"	Str.						12											
A511	2	5'-2"	Str.						11											
A512	2	8'-0"	2	3'-4"	4'-8"	0	4 1/2	0	17											
A513	2	7'-5"	2	2'-9"	4'-8"	0	4 1/2	0	15											
A514	6	7'-0"	Str.						44											
A515	2	4'-0"	Str.						8											
A516	2	4'-7"	Str.						10											
																			Total	7593

* Varies 2'-3" to 3'-9". 4 Sets of 4 bars, vary each by 6"



REINFORCING STEEL SAMPLES: Refer to CMS Sections 106.03, 700, 709.01 through 709.05 and 709.08. Sufficient additional reinforcing steel shall be provided for sampling. Random samples shall be replaced in the structures by the additional steel, spliced in accordance with 509.08.

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ABUTMENTS
BRIDGE NO. MED-42-0945
OVER CAMEL CREEK

MEDINA COUNTY STA. 499+19.33 #0 STA. 499+60.67 U.S. RT. 42

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
DBC	DKS	RLE	DWS	WRH	9/21/81	

SUPERSTRUCTURE NOTES

PRESTRESSED CONCRETE BOX BEAMS

CONCRETE STRESSES:
 Min. Concrete Strength at 28 days $f'c = 5500$ psi.
 Min. Concrete Strength at time of initial prestress $f'ci = 4000$ psi.

PRESTRESSING STRANDS: $\frac{1}{2}$ " dia. 270^k seven wire, uncoated, stress-relieved strand. $A_s = 154$ sq. in.
 Initial Tension = 28,900 lbs. per strand.
 Tension at release = 26,600 lbs. per strand (Assumed).
 Final Tension = 21,700 lbs. per strand after all losses (Assumed)

REINFORCING STEEL:
 2" of concrete cover, measured from the top of the box beams, shall be provided over the upper steel. Beam manufacturer's shop drawings shall show complete details of box beam reinforcing.

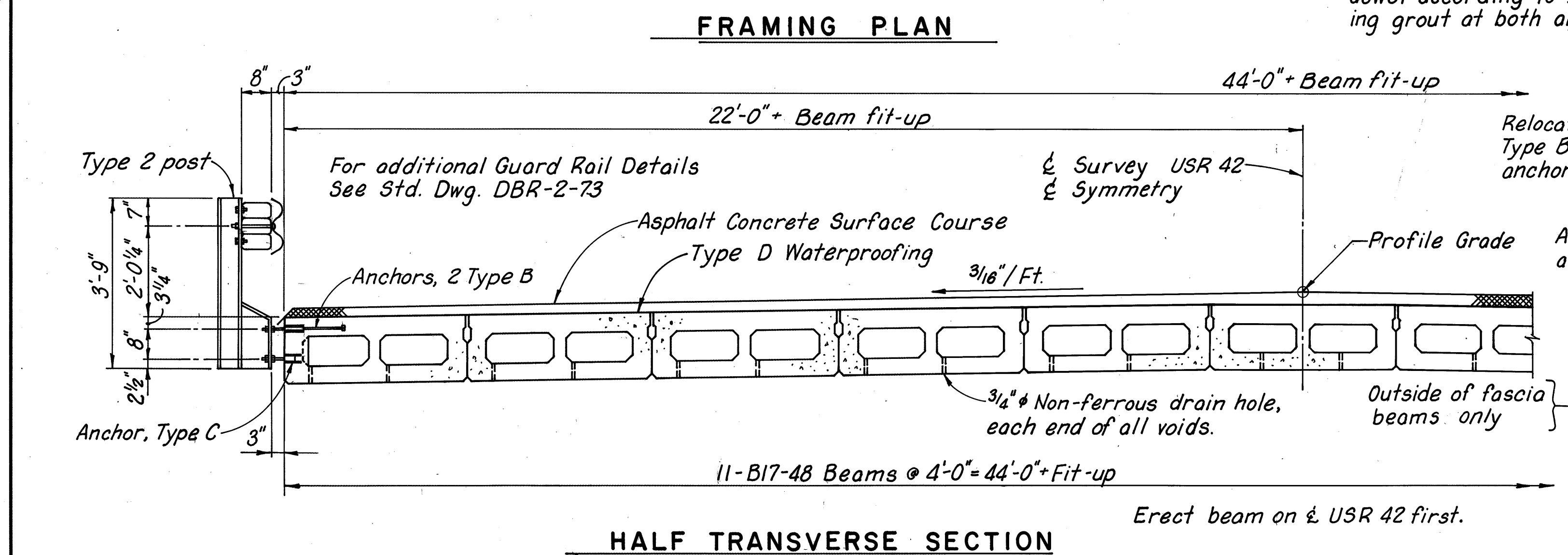
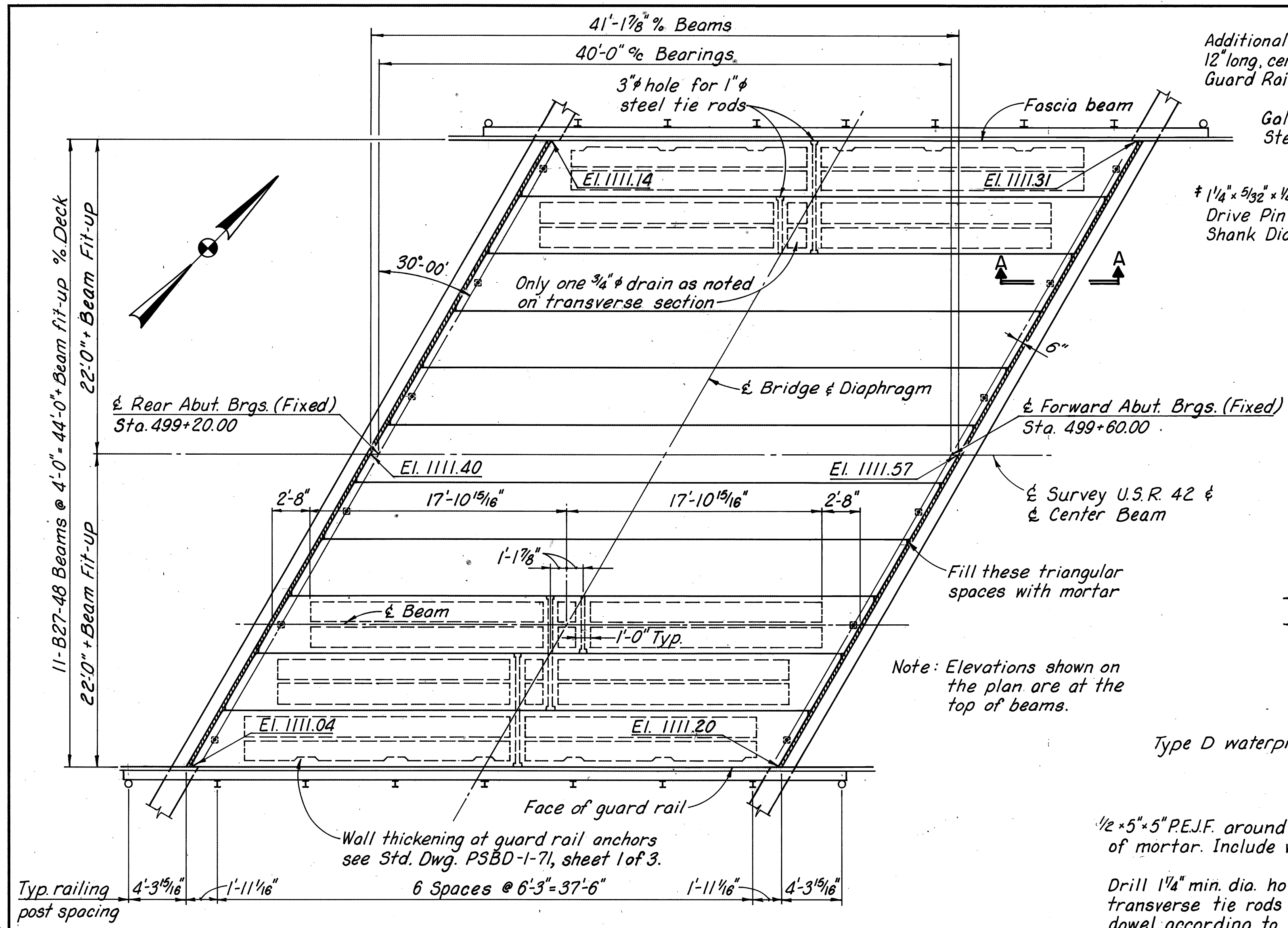
ADDITIONAL PRESTRESSED BEAM NOTES & DETAILS:
 See Standard Drawing PSBD-1-81, sheets 1, 2 and 3 of 4 for Beam Lifting Inserts, Details and Reinforcement of Beam Ends, End Details of Transverse Tie Rod Anchorage, Normal Crown Treatment at Roadway, and Beam Dimensional Tolerances.
 See Standard Drawing PSBD-1-81, sheet 1 of 4 for notes on Transverse Tie Rods, Galvanizing, Anchor Dowels, Non-shrinking Mortar and Grout, and Mortaring of Shear Keys.

CAMBER: Calculated camber at time of paving including allowance for camber growth due to creep is $\frac{1}{8}$ ". Calculated deflection due to weight of surface course and railing is $\frac{1}{8}$ ".
 Net final camber of beams is $\frac{1}{2}$ ". This is $\frac{1}{2}$ " in excess of the amount required to place the top of the beam parallel to profile grade. This excess amount shall be compensated for by thickening the 403 leveling course from $\frac{1}{2}$ " at center of spans to 3" at end of spans.

RAILING: See Standard Drawing DBR-2-73

ABUTMENT BEARING DETAILS: See Sheet 3/4. Two $\frac{1}{8}$ " x 4" x 10" preformed bearing pads per beam shall be provided for use as shims where needed for proper seating of the prestressed beams.

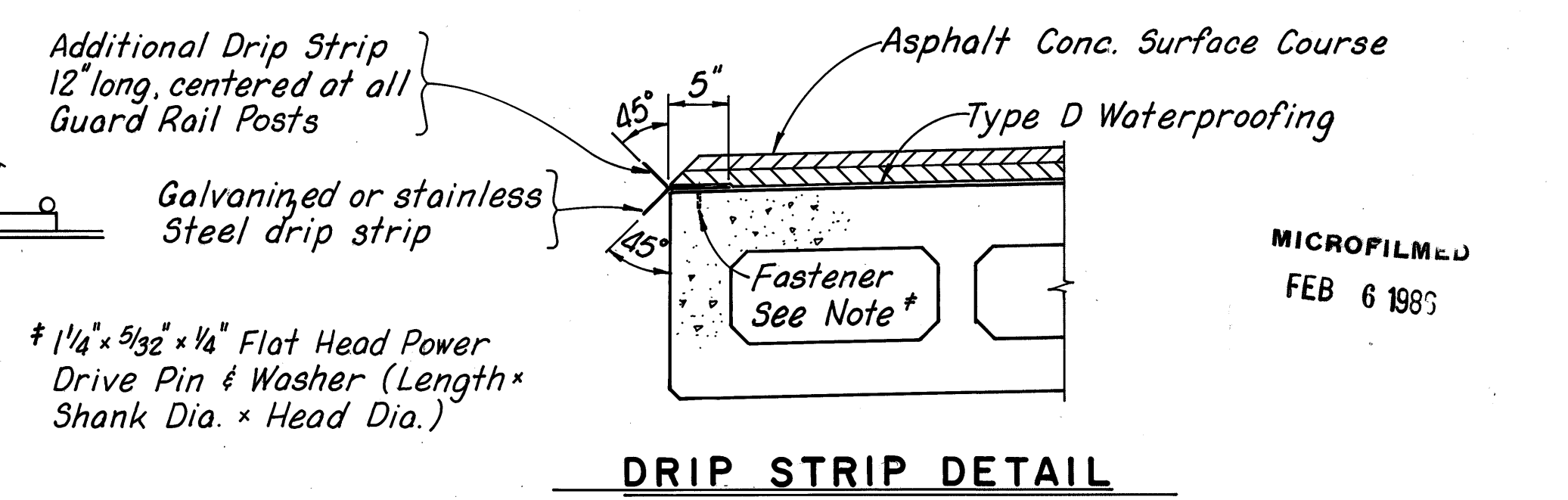
GENERAL NOTES: See Sheet 2/4



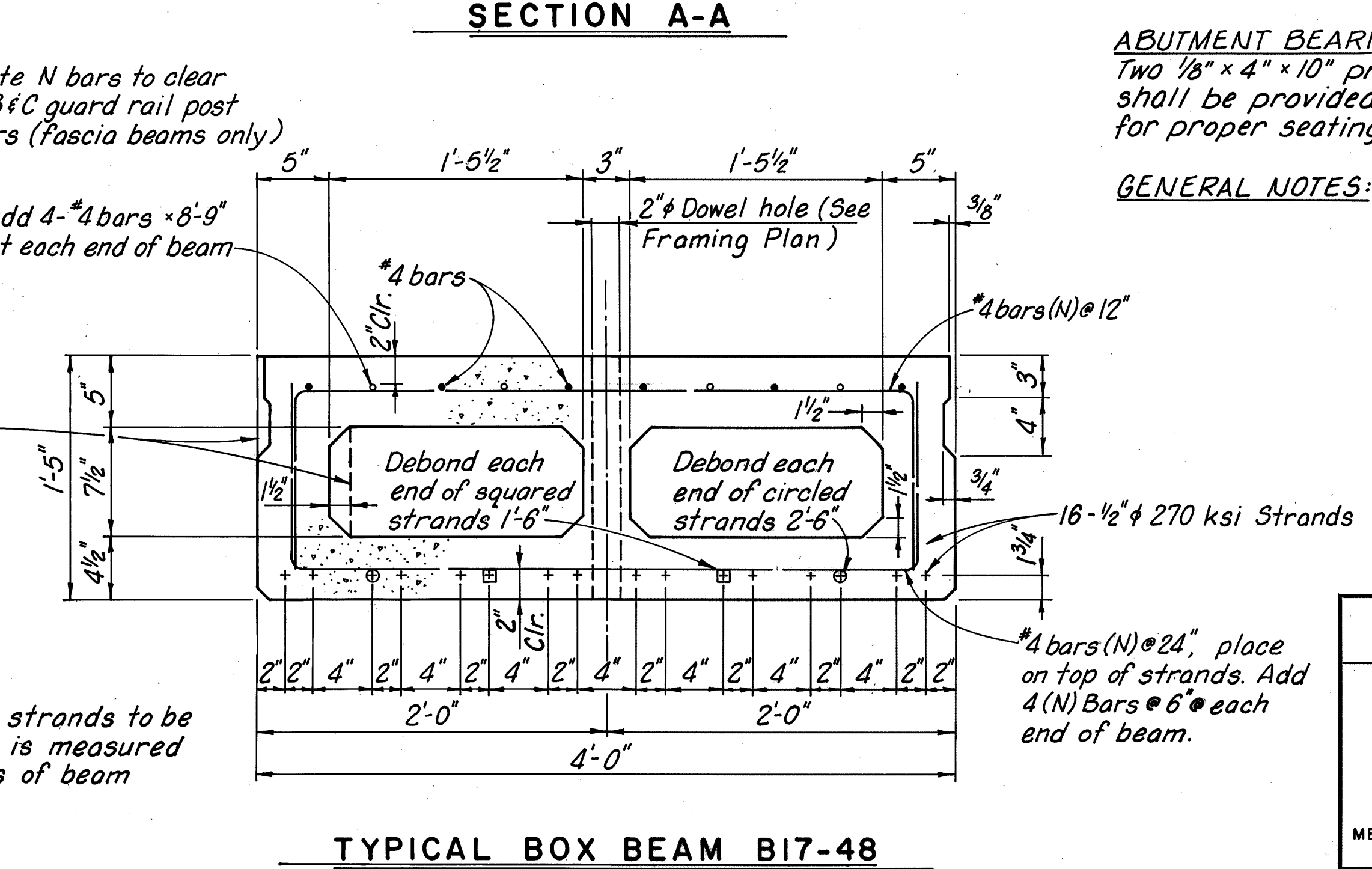
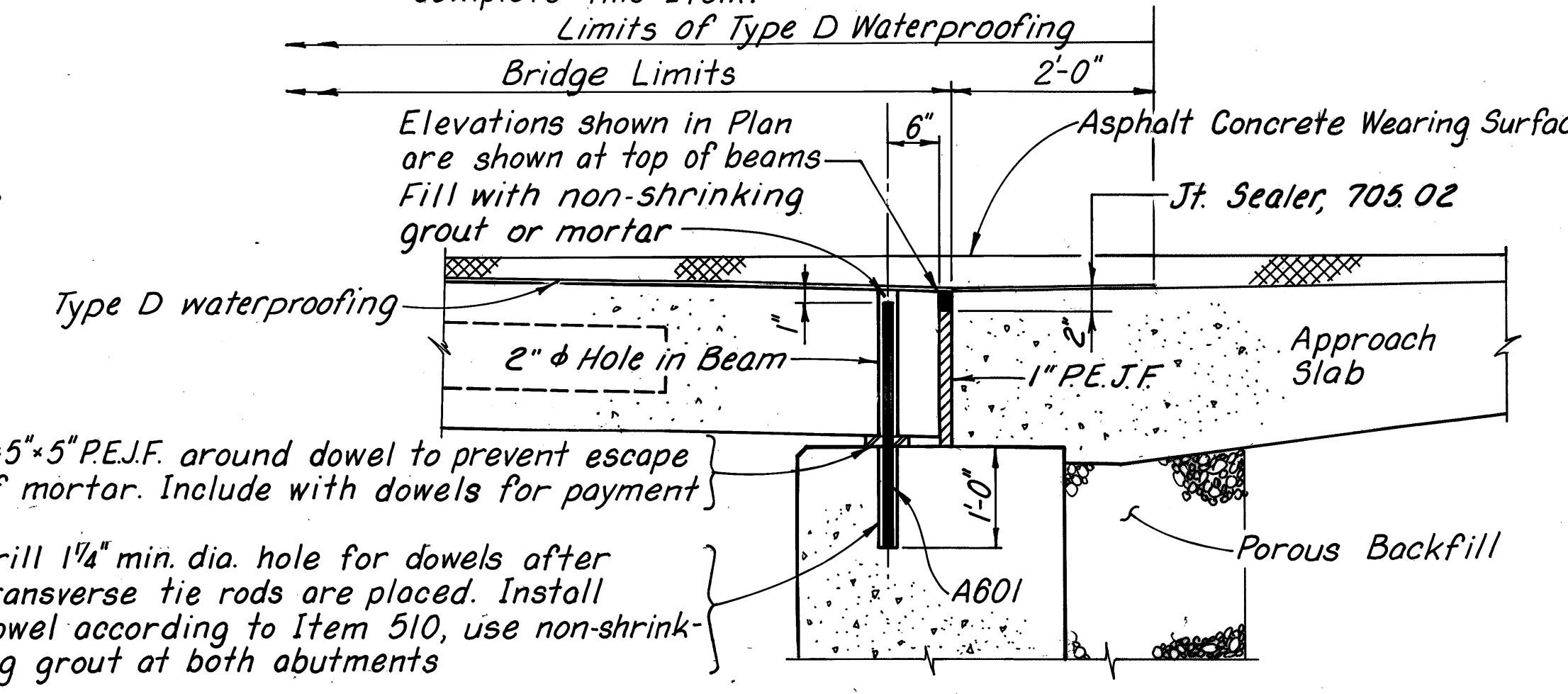
ASPHALT CONCRETE SURFACE COURSE shall consist of a variable thickness of 403 and a 1" thickness of 404. The 403 shall be placed in two operations. The first course shall be of $\frac{1}{2}$ " uniform thickness. The second course shall be feathered to place the surface parallel to and 1" below final pavement surface elevation.

BEAM REINFORCING					
MARK	TYPE	A	B	C	BENDING DIAGRAM
N	I	3'-7"	1'-3"	1'-3"	

Length of strands to be debonded is measured from ends of beam



Prior to applying Type D Waterproofing a bent steel drip strip shall be installed along the edges of the deck as shown. The strips shall be fastened at 1'-6" maximum with power driven pins* or #10 galvanized screws and expansion anchors subject to the approval of the Engineer. The strips shall be placed the full length of the deck, ending at the face of abutment wingwall. Where splices are required a 3" (minimum) lap shall be used with the fastener through the lap. Steel for galvanized strips shall be 8" x 0.105" and shall meet the requirements of ASTM A568. Galvanizing shall be in accordance with 711.02. Stainless steel shall be 20 gauge ASTM A167, Type 304, Mill Finish. Payment shall be made at the contract bid price for Item Special, Square Feet Steel Drip Strip, which shall include all material, labor, tools and incidentals necessary to complete this item.



4 / 4

ENGINEERING ASSOCIATES LTD.
 CONSULTING ENGINEERS
 700 WINKLER DR. WOOSTER, OHIO

SUPERSTRUCTURE
 BRIDGE NO. MED-42-0945
 OVER CAMEL CREEK

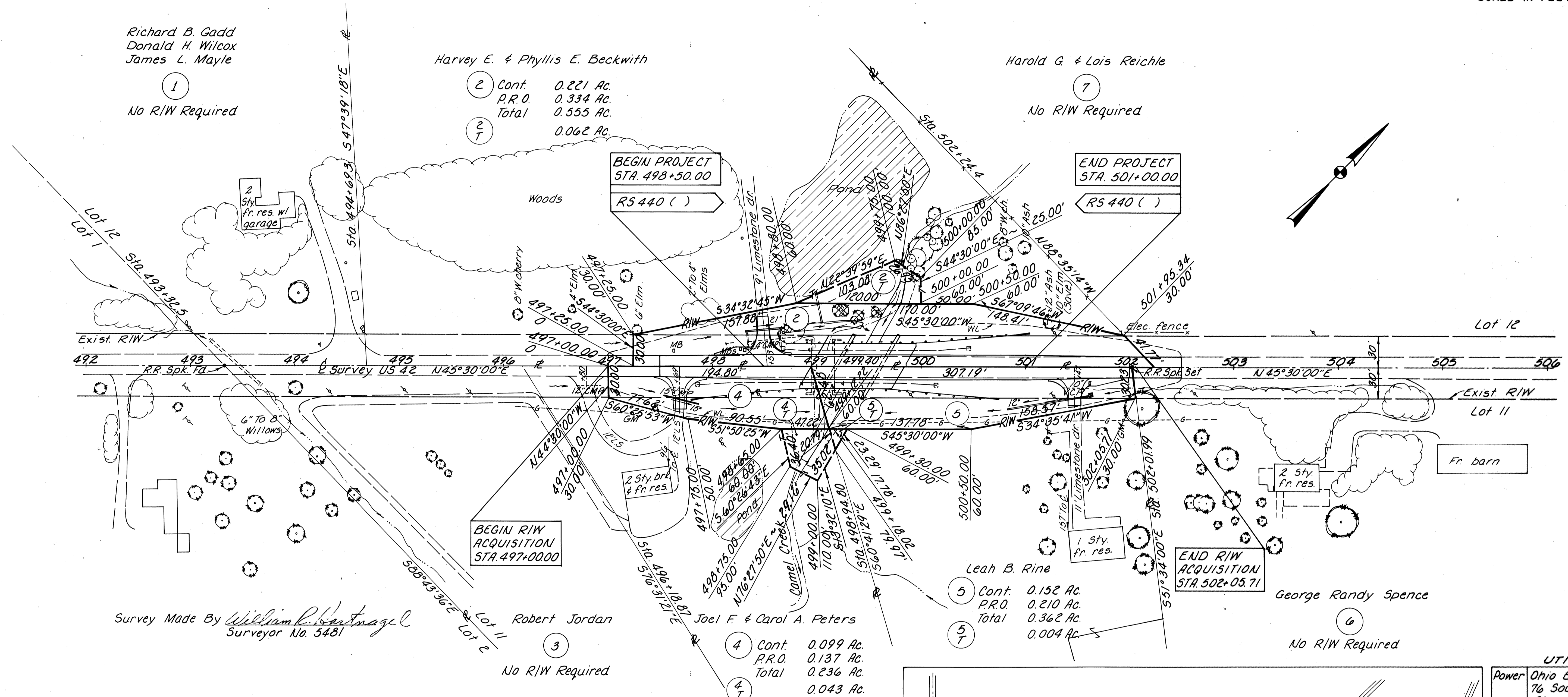
MEDINA COUNTY U.S. RT. 42
 STA. 499+19.33 TO STA. 499+60.67

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
DBC	DKS	RLE	DWS	WRH	9/21/81	

MEDINA COUNTY LAFAYETTE TOWNSHIP
 T-2N R-15W
 TRACT 3 LOTS 11 & 12

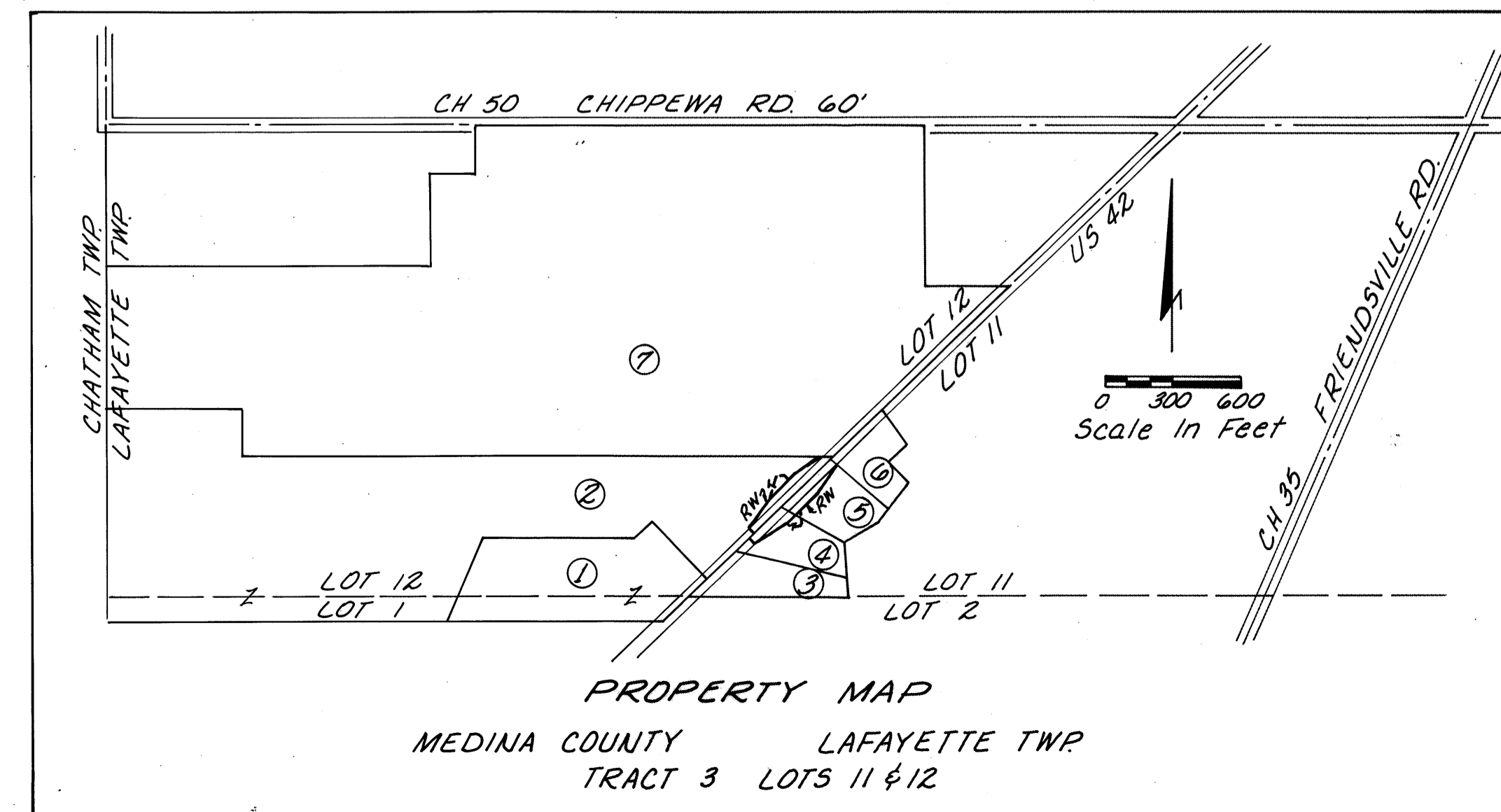
CALC BY P.L.D. DATE 2-27-81	MED - 42 - 9.44	OHIO	15
CHKD BY WRH DATE 2-28-81		FHWA REGION 5	15
FEDERAL PROJECT BRS 440 ()	STATE PROJECT 03379 (0)		1

RIGHT-OF-WAY PLAN
 SCALE IN FEET



SUMMARY OF ADDITIONAL RIGHT OF WAY REQUIRED

PARCEL NUMBER	OWNER	DEED BOOK	DEED PAGE	DEED DATE	TOTAL AREA	TOTAL PRO TAKE	NET TAKE	NET RES. LI.	NET RES. RI.	BLDG'S ACQ'D	REMARKS	TYPE FUNDS
1	RB Gadd, D.H. Wilcox, J.L. Mayle	518	412	2-23-79	No	No	R/W Required					State
2	Harvey E. & Phyllis E. Beckwith	403	410	3-31-71	41.70 A.	5.11 A.	5.55 A.	3.34 A.	2.21 A.	40.96 A.		
2T						0.062 A.	0	0.062 A.			Channel Cleanout	
3	Robert Jordan	508	68	6-6-78	No	No	R/W Required					
4	Joel F. & Carol A. Peters	460	633	9-4-74	180.4 A.	1.87 A.	2.36 A.	1.37 A.	0.99 A.	151.8 A.		
4T						0.043 A.	0	0.043 A.			Channel Cleanout	
5	Leah B. Rine	427	250	6-21-72	218.5 A.	2.10 A.	3.62 A.	2.10 A.	1.52 A.	182.3 A.		
5T						0.004 A.	0	0.004 A.			Channel Cleanout	
6	George Randy Spence	516	546	12-22-78	No	No	R/W Required					
7	Harold G. & Lois Reichle	230	405	11-23-55	No	No	R/W Required					



UTILITY OWNERS

Power	Ohio Edison Co. 76 South Main St. Akron, Ohio 44308 Ph. 216-384-5234
Phone	General Telephone Co. of Ohio 108 W. Washington St. Medina, Ohio 44256 Ph. 216-725-8411
Gas	Columbia Gas Transmission Corp. P.O. Box 1273 Charleston, W. VA. 25314 Ph. 304-346-0951

TYPE FUNDS: State

REV DATE	DESCRIPTION
	PLAN COMPLETED: 7/16/81

RIGHT-OF-WAY PLAN

GEOLOGY OF THE SITE

The structure site is located on a gently rolling, glaciated plateau, in an area where glacial-derived material overlies bedrock, of Mississippian age.

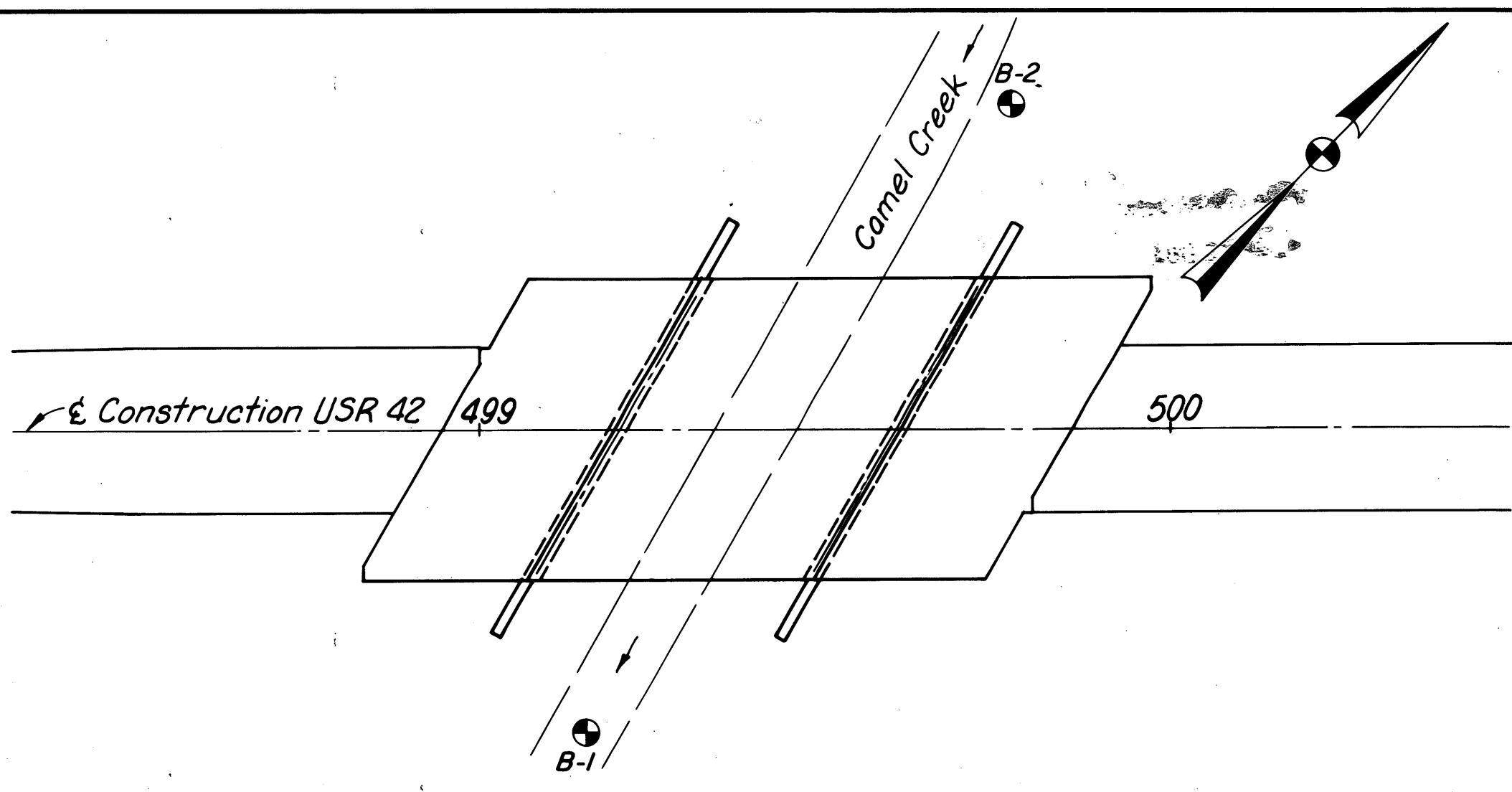
EXPLORATION

The exploration consisted of two drive sample borings made by means of a mechanically-powered hollow stem auger on February 7, 1980.

INVESTIGATIONAL FINDINGS AND OBSERVATIONS

The borings disclosed subsoils consisting of medium dense to stiff, moist clayey silt with traces of sand and gravel, sandstone and shale fragments in the upper 8 to 12 feet of each boring. Directly below this material to a depth of approximately 40 feet below the surface, the soil consists of very dense, moist, silty sand and gravel. Neither boring penetrated bedrock surface. Borings B-1 and B-2 were terminated at 40 to 41.9 foot depths, elevations 1065.1 and 1064.7 after penetrating 30 feet of material, requiring an excess of 30 blows per foot in the Standard Penetration Test.

Free water was observed and measured in boring B-1 at 5.5, 14.5, and 27.0 foot depths, elevations 1099.6, 1090.6, and 1078.1 respectively and in boring B-2 at 5.0 and 19.5 foot depths, elevations 1101.6 and 1087.1.



GENERAL INFORMATION

Drive Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. sampler, at 2-1/2 and/or 5-foot depth intervals, driven by means of a 140-pound drop-hammer with a free fall of 30 inches. For each sample obtained the sampler is driven a total of 18-inches except when a condition of refusal is met before obtaining a penetration of 18-inches. The blow count is recorded for each 6-inch increment. The first 6-inches of drive is considered setting the sampler while the sum of the number of blows required to drive the sampler the final two increments is designated the Standard Penetration Count. This count is denoted by the letter N.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in three 6-inch increments, field sample number, and sample description.

LEGEND

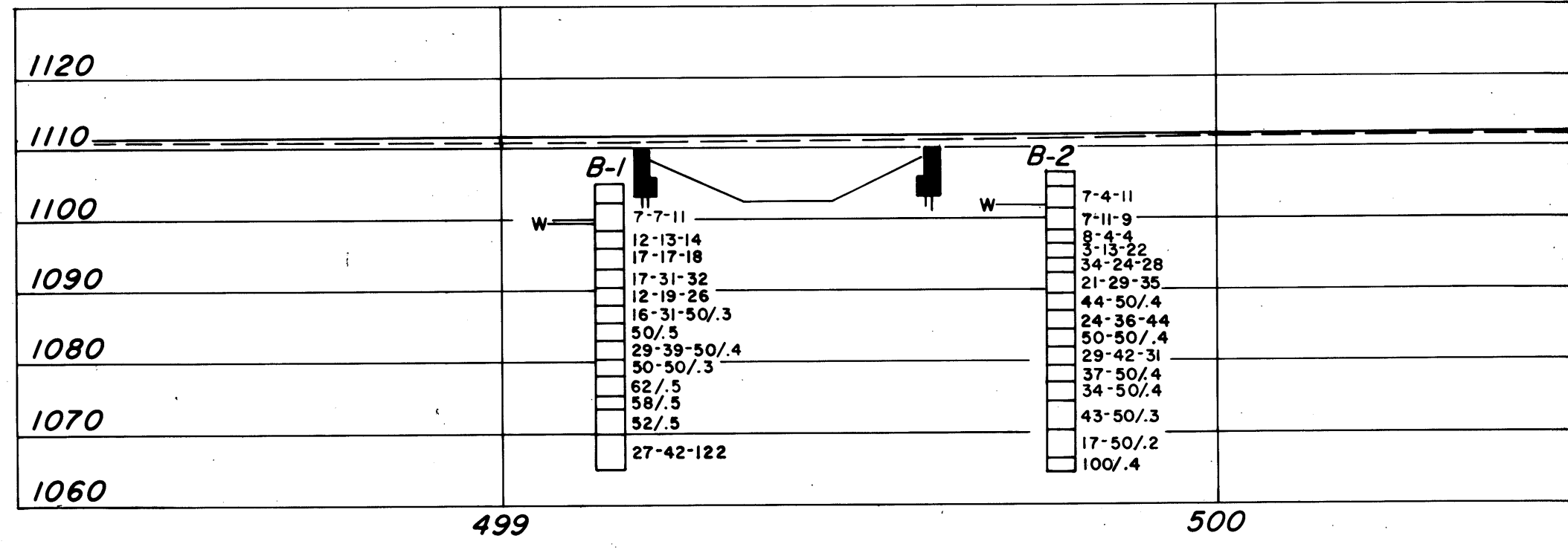
- Core Boring Location - Plan View
- ⊥ Capped Pile Footing
- H Horizontal Bar on Boring Log Indicates the Depth (Median) at which the Sample was Taken.
- W Indicates Free Water Elevation.
- X/Y/Z Figures Beside the Boring Log in Profile Indicate the Number of Blows for Standard Penetration Test.
 X = Number of Blows for First 6 inches.
 Y = Number of Blows for Second 6 inches.
 Z = Number of Blows for Third 6 inches.

3-85

LOG OF BORING
 DATE STARTED 2-7-80 SAMPLER TYPE S.S. DIA. 2" O.D. WATER ELEV. 1100.1
 DATE COMPLETED 2-7-80 CASING LENGTH DIA. SURFACE ELEV. 1105.1
 BORING NO. B-1 STATION & OFFSET 499+15, 44' RT.

ELEV.	DEPTH	STD. PEN. BLOWS / 6"		REC. FT.	DESCRIPTION	SAMPLE NO.	PHYSICAL CHARACTERISTICS											
		%	%				%	%	%	%	LL	P.I.	W.C.					
1105.1	0																	
1102.6	2				Brown clayey silt with minor gravel		VISUAL											
1098.6	4	7-7-11	18	.6'	Gray clayey silt with trace of sand and sandstone fragments. Medium dense, moist.	1	VISUAL											
1096.1	6																	
1096.1	10	12-13-14	27	1.2'	Gray silty clay, trace of sand and gravel. Stiff, moist.	2	VISUAL	20	9	8	48	15	24	8				
1093.1	12	17-17-18	35	.6'	Gray clayey silt, trace of sand and gravel. Dense, moist.	3	VISUAL											
1090.6	14	17-31-32	63	1.0'	Gray sandy silt, trace of clay and gravel, sandstone fragments. Very dense, moist.	4	VISUAL											
1088.1	16	12-19-26	45	1.1'	Gray silty sand with minor gravel. Dense, wet.	5	VISUAL											
1085.6	18	16-31-50	81+	1.2'	Gray silty sand and gravel. Very dense, wet	6	VISUAL											
1083.1	20	50/.5	50+	.5'	Gray silty sand and gravel. Very dense, moist	7	VISUAL											
1080.6	22	29-39-50	89+	.8'	Gray clayey silt with minor sand and gravel. Very dense, moist.	8	VISUAL											
1078.1	24	50-50/.3	80+	.7'	Brown sand and gravel with trace of silt. Very dense, moist.	9	VISUAL											
1075.6	26	62.5	62+	.5'	Brown sand and gravel with minor silt. Very dense, wet.	10	VISUAL											
1073.6	28	58/.5	58+	.5'	Brown and gray sand and gravel with trace of silt. Very dense, wet.	11	VISUAL											
1070.1	30	52/.5	52+	.4'	Gray sandy silt with gravel and sandstone fragments. Very dense, moist.	12	VISUAL											
1065.1	32	27-42-122	164	.5'	Gray sandy silt with gravel and minor clay (sandstone fragments). Very dense, moist.	13	VISUAL											

BORING COMPLETED



LOG OF BORING
 DATE STARTED 2-7-80 SAMPLER TYPE S.S. DIA. 2" O.D. WATER ELEV. 1103.1
 DATE COMPLETED 2-7-80 CASING LENGTH DIA. SURFACE ELEV. 1106.6
 BORING NO. B-2 STATION & OFFSET 499+78, 47' L.T.

ELEV.	DEPTH	STD. PEN. BLOWS / 6"		REC. FT.	DESCRIPTION	SAMPLE NO.	PHYSICAL CHARACTERISTICS											
		%	%				%	%	%	%	LL	P.I.	W.C.					
1106.6	0																	
1104.6	2				Brown clayey silt with minor sand		VISUAL											
1101.6	4	7-4-11	15	.4'	Brown silty clay with sandstone fragments. Medium dense, moist.	1	VISUAL											
1098.6	6	7-11-9	20	.4'	Gray silty clay with shale fragments. Stiff, wet.	2	VISUAL											
1096.6	8	8-4-4	8	.6'	Gray silty sand with minor clay and gravel. Loose, wet.	3	VISUAL											
1094.6	10	3-13-22	35	.5'	Gray silty sand and gravel with minor clay. Dense, wet.	4	VISUAL											
1092.6	12	34-24-29	52	.6'	Gray gravel and shale fragments with trace of sand, clay, Very dense, moist.	5	VISUAL											
1089.6	14	21-29-35	64	.6'	Gray gravel with minor silt, sand and clay. Very dense, wet.	6	VISUAL											
1087.1	16	44-50/.4	94+	.8'	Brown gravel with sand, trace of silt. Very Dense, moist.	7	VISUAL											
1084.6	18	24-36-44	80	1.4'	Brown sand and silt with minor gravel. Very dense, wet.	8	VISUAL											
1082.1	20	50-50/.4	100+	1.2'	Brown sandy silt with trace of gravel. Very dense, moist.	9	VISUAL											

BORING COMPLETED

B-2 (Cont'd.)

ELEV.	DEPTH	STD. PEN. BLOWS / 6"		REC. FT.	DESCRIPTION	SAMPLE NO.	PHYSICAL CHARACTERISTICS											
		%	%				%	%	%	%	LL	P.I.	W.C.					
1082.1	26	29-42-31	73	1.0'	Brown and gray gravel with minor sandy silt. Very dense, wet.	10	VISUAL											
1079.6	28	37-50/.4	87+	.9'	Brown sand and gravel with minor silt. Very dense, wet.	11	VISUAL											
1077.1	30	34-50/.4	84+	.8	Brown fine sandy silt. Very dense, wet.	12	VISUAL											
1074.6	32	43-50/.3	93+	.8'	Brown silty sand. Very dense, wet.	13	VISUAL											
1070.6	34	17-50/.2	67+	.6'	Brown silty sand with minor gravel and trace of clay. Very dense, moist.	14	VISUAL											
1064.7	36	100/.4	100+	0	Brown silty sand with minor gravel and trace of clay. Very dense, moist.	15	VISUAL											

ENGINEERING ASSOCIATES LTD.
 CONSULTING ENGINEERS
 700 WINKLER DR. WOOSTER, OHIO

STRUCTURE FOUNDATION INVESTIGATION
 BRIDGE NO. MED - 42-0945
 OVER CAMEL CREEK

MEDINA COUNTY STA. 499+19.33 TO STA. 499+60.67 U.S. RT. 42

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
1111	RLE		DBC	WRH	9/21/81	