FINAL REPORT ROADWAY EXPLORATION REPORT CUY-14-6.93 CUYAHOGA COUNTY, OHIO PID#: 104132

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NEAS PROJECT 18-0023

February 29, 2024



TABLE OF CONTENTS

1. INTE	RODUCTION	2
	GENERAL	
2. GEO	LOGY AND OBSERVATIONS OF THE PROJECT	2
2.1.	GEOLOGY AND PHYSIOGRAPHY	2
	HYDROLOGY/HYDROGEOLOGY	
	MINING AND OIL/GAS PRODUCTION	
2.4.	HISTORICAL RECORDS AND PREVIOUS PHASES OF PROJECT EXPLORATION	3
	FIELD RECONNAISSANCE	
2.5.1.		
2.5.2.	State Route 14	4
2.5.3.		
2.5.4.		
	TECHNICAL EXPLORATION	
	EXPLORATION PROGRAM	
3.2.	LABORATORY TESTING PROGRAM	
3.2.1.	- · · · · · · · · · · · · · · · · · · ·	
3.2.2.		
	DINGS	
	SUBGRADE CONDITIONS	
4.1.1.		
4.1.2.	- · · · · · · · · · · · · · · · · · · ·	
4.1.3.	- · · · · · · · · · · · · · · · · · · ·	
4.1.4.		9
	LYSES AND RECOMMENDATIONS	
	SUBGRADE ANALYSIS	
5.1.1.	O Company of the comp	
	Unsuitable Subgrade	
	.2.1. Rock	
	.2.2. Prohibited Soils	
	.2.3. Weak Soils	
	2.2.4. High Moisture Content Soils	
	STABILIZATION RECOMMENDATIONS	
5.2.1. 5.3.		
	EMBANKMENT STABILITY ANALYSIS	
	EMBANKMENT CONSTRUCTION RECOMMENDATIONS	
o. QUA	LIFICATIONS	.14
	LIST OF TABLES	
TABLE 1:	PAVEMENT DESIGN VALUES	.10
TABLE 2:	WEAK SOIL LOCATIONS SUMMARY	
TABLE 3:	HIGH MOISTURE CONTENT SOIL LOCATIONS	
TABLE 4:	SUMMARY OF STABILIZATION	.12

LIST OF APPENDICES

APPENDIX A: SOIL PROFILE SHEETS

APPENDIX B: BORING LOGS

APPENDIX C: GEOTECHNICAL BULLETIN 1 (GB1) ANALYSIS SPREADSHEETS

APPENDIX D: EMBANKMENT STABILITY OUTPUTS



1. INTRODUCTION

1.1. General

National Engineering & Architectural Services, Inc. (NEAS) presents our Roadway Exploration Report for the CUY-14-6.93 project (PID 104132) along portions of State Route 14 (SR-14) / Broadway Avenue (Ave), County Road 24 (CR-24) / Henry Street (St), Chaincraft Road (Rd) and Old Broadway Ave in the City of Garfield Heights, Cuyahoga County, Ohio. The overall project objective is to: 1) replace the existing bridge over Chaincraft Rd, Norfolk Southern Railway (Rwy) and Wheeling & Lake Erie Rwy (CUY-14-0693) with a new shortened structure on a new alignment; 2) remove the existing bridge (CUY-CR24-0062) over the existing culvert CUY-CR00240-00.610; and, 3) replace a segment of the existing culvert directing Mill Creek under the existing bridge CUY-CR24-0062 and Chaincraft Rd. This report presents a summary of the project encountered surficial and subsurface conditions and our recommendations for subgrade stabilization, pavement design parameters, and embankment stability for the realignment and/or full-depth replacement of SR-14, Henry St, Chaincraft Rd and Old Broadway Ave. In general, the pavement subgrade analysis and recommendations presented are in accordance with ODOT's Geotechnical Bulletin 1 (GB1) (ODOT [1], 2022) and Pavement Design Manual (PDM) (ODOT PDM, 2021).

The exploration for the referenced project was conducted in general accordance with Barr Engineering, Inc. DBA National Engineering and Architectural Services Inc.'s (NEAS) proposal to AECOM, dated June 7, 2021 and with the provisions of ODOT's *Specifications for Geotechnical Explorations* (SGE) (ODOT, 2021).

The scope of work performed by NEAS as part of the referenced project included: a review of published geotechnical information; performing 22 total test borings; laboratory testing of soil samples in accordance with the SGE; performing geotechnical engineering analysis to assess subgrade stabilization requirements, pavement design parameters and embankment construction; and, development of this summary report.

2. GEOLOGY AND OBSERVATIONS OF THE PROJECT

2.1. Geology and Physiography

The project site is located within the Galion Glaciated Low Plateau (ODGS, 1998). This area is characterized as rolling upland mantled with thin to thick drift and is transitional between the gently rolling Till Plain and the hilly Glaciated Allegheny Plateau with the overall area ranging in elevation from 800 ft to 1400 ft, with moderate relief (100 ft). The geology is described as medium- to low-lime Wisconsinan-age till over Mississippian-age shales and sandstone.

The geology underlying the majority of the project site is mapped as up to 70 ft of Wisconsinan-age sand and gravel thinning to an average thickness of 40 ft near the eastern limits of the project site, all over Mississippian-age sandstone and shale bedrock (ODGS, 2002). A portion of the eastern part of the project site is mapped as an average of 40 ft of Wisconsinan-age till overlying the natural sand and gravel followed by about 10 ft of till then bedrock. On the southwestern part of the project (underlying Henry St), the geology is mapped as solely till, with an average thickness of 30 ft, overlying the bedrock. The sand and gravel soils mapped at the project site are generally described as interbedded, well to moderately sorted sand and gravel commonly containing thin, discontinuous layers of silt and clay. The sand and gravel are characterized as finely stratified to massive, may be cross-bedded and locally may contain organics. The



till at the project site is generally described as an unsorted mix of clay, silt, sand, gravel and boulders that may contain silt, sand and gravel lenses.

Bedrock underlying the project site has been mapped as Berea Sandstone and Bedford Shale, undivided, based on the Geologic Units Map of Ohio (USGS & ODGS, 2006). The sandstone at the site is described as brown, weathering light brown to reddish brown, thin to thick bedded with planar to lenticular bedding. The shale at the site is described as gray to brown in color, locally reddish brown, thin to medium bedded with planar to lenticular bedding as well. Based on the ODNR bedrock topography map of Ohio, the bedrock elevation at the project site can be expected at an approximate elevation of 800 ft above mean sea level (amsl), putting bedrock at depths between 15 ft and 50 ft below ground surface (bgs).

The soils underlying the project site have been mapped (Web Soil Survey) by the Natural Resources Conservation Service as being a combination of Urban land, Loudonville-Urban land complex and pits/quarry. Urban land is mapped underlying the existing Henry St as well as SR-14 east of Chaincraft Rd. The Loudonville-Urban land complex is mapped underlying the majority of the project area west of Chaincraft Rd as well as the area underlying the proposed bridge location, while pits/quarry are mapped underlying the northern most portion of the project encompassing the majority of the proposed RW-G site. Urban land is land that has been altered or obscured by urban work and structures. Soils within these areas can be non-native human-transported material, human-altered material, or minimally altered or intact native soils. Urban land soils are not rated for local roads. The Loudonville series is described as moderately deep, well drained soils with moderate permeability formed in loamy till and underlain by sandstone or siltstone. Soils in the Loudonville series classify as both cohesive and non-cohesive A-4 soils and cohesive A-6 and A-7 soils according to the AASHTO method of soil classification (USDA, 2019).

2.2. Hydrology/Hydrogeology

Groundwater elevations at the project site are anticipated to be near elevations consistent with that of the immediately adjacent Mill Creek (between approximate elevations 805 and 807 ft amsl) as it is the most dominant hydraulic influence in the vicinity of the project site. The water level of Mill Creek may be representative of the local groundwater table although perched groundwater systems may exist with the presence of fine-grained soils making it difficult for groundwater to permeate to the natural phreatic surface.

Areas in the eastern portion of the project are located within a 1% Annual Chance Flood Hazard area. Based on available mapping by the Federal Emergency Management Agency's (FEMA) National Flood Hazard mapping program (FEMA, 2016) the 1% Annual Chance Flood elevation is about 822 to 823 ft amsl within the flood hazard area).

2.3. Mining and Oil/Gas Production

No abandoned mines are noted on ODNR's Abandoned Underground Mine Locator within the immediate vicinity of the project site (ODNR [1], 2016).

No oil or gas wells are noted on ODNR's Ohio Oil & Gas Locator within the immediate vicinity of the project site (ODNR [2], 2016).

2.4. Historical Records and Previous Phases of Project Exploration

The following plans were available for review and evaluation for this report:



• Bridge Foundation Investigation sheets and boring logs as part of ODOT bridge rehabilitation project CUY-14-06.99 Broadway Avenue Bridge No. 123, prepared by the Mason, Sandefur & de Verteuil, Inc., dated August 23, 1983;

Historical soil borings associated with the above referenced plans were reviewed, however, were not utilized for our analysis, and therefore, are not referenced or presented within this report or soil profile sheets.

2.5. Field Reconnaissance

A field reconnaissance visit for the overall project area was conducted between August 6, 2021 and August 7, 2021 along SR-14, CR-24 (Henry St), and Chaincraft Rd. Site conditions, including the conditions of existing pavement, embankments and structures, were noted and photographed during the visit. Photographs of notable distress and a summary of our observations by roadway segment are provided below.

2.5.1. Land Use and Cover

The land use of most of the project area consists of recreational (park) and industrial properties. Industrial land uses of the area surrounding the project include: 1) a recycling facility; 2) a precast concrete facility; and, 3) a trucking/logistic facility.

2.5.2. State Route 14

A majority of the project portion of SR-14 is currently bridge deck (existing Bridge CUY-14-0693) with the exception of the first and last approximate 250 ft which is pavement supported on grade. The pavement condition along the portions of SR-14 supported on grade was observed to be fair to good condition with minor signs of weathering and surface wear (Photograph 1). Occasional low to moderate severity longitudinal, edge and transverse cracking was observed along these sections. The exception to this is the sections of pavement near existing utilities and manhole. Pavement in these areas was observed to be in poor condition with noted patching and pavement settlement. The first 250 ft of SR-14 is supported on embankments with side slopes of about 2 horizontal to 1 vertical (2H:1V). The referenced slopes were generally vegetated with small trees, brush and grass with no apparent signs of instability observed during the visit. With respect to drainage, the roadway appeared to be well-drained with no observable signs of ponding or standing water.







2.5.3. Henry Street

The condition of the pavement along the project portion of Henry St (approximately 240 ft of pavement) was observed to be poor with major signs of weathering and surface wear. Frequent, moderate to high severity longitudinal, edge and transverse cracking was observed along this section as well as high severity rutting, patching and crack sealing deficiencies. The project portion of Henry St is supported on an embankment however, due to heavy vegetation along the existing slope, the slope condition and grading were difficult to discern. In general, the pavement appeared to be well-drained with no observable signs of ponding or standing water on the roadway.



Photograph 2: Henry St pavement condition

2.5.4. Chaincraft Road

The pavement along Chaincraft Rd consists of both asphalt and concrete pavement sections which were generally observed to be in fair to poor condition with moderate signs of weathering and surface wear. Within the asphalt pavement areas, frequent low to moderate severity longitudinal and transverse cracking were observed as well as occasional low to moderate severity potholes and crack sealing deficiencies (Photograph 3). The concrete pavement areas were observed to have frequent, moderate severity edge cracking and joint separation (Photograph 4). With respect to drainage, both pavement appeared to be well-drained with no observable signs of ponding or standing water on the roadway with the exception of water observed within pavement cracks and potholes.



Photograph 3: Asphalt portion of Chaincraft Rd



Photograph 4: Concrete portion of Chaincraft Rd



3. GEOTECHNICAL EXPLORATION

3.1. Exploration Program

The subsurface exploration was conducted by NEAS between August 2, 2021 and September 13, 2021 and included 22 borings drilled to depths between 7.5 and 61.5 ft bgs. The boring locations were selected by NEAS in general accordance with the guidelines contained in the SGE and preliminary project plans available at the time of drilling with the intent to evaluate subsurface soil and groundwater conditions. Borings were typically located within the planned roadway/subgrade improvement areas and/or at or near proposed structure locations that were not restricted by underground utilities or dictated by terrain (i.e., steep embankment slopes). Target boring locations were located in the field by NEAS prior to drilling utilizing handheld GPS equipment. If the actual drilled location was relocated for drilling purposes, the asdrilled project boring location and corresponding ground surface elevation was again surveyed in the field following drilling. Each individual project boring log (included within Appendix B) includes the recorded boring latitude and longitude location (based on the surveyed Ohio State Plane North, NAD83, location) and the corresponding ground surface elevation. The boring locations are depicted on the Soil Profile Sheets provided in Appendix A. It should be noted that a number of borings were drilled for project structures but can serve as both structure and roadway borings.

Borings were drilled using either a CME 55, CME 75 or CME 55X truck- or track-mounted drilling rig utilizing 3.25-inch (inner diameter) hollow stem augers. Soil samples for subgrade borings were typically recovered continuously to a depth of 7.5 ft bgs, while samples for joint subgrade/structure borings were typically recovered continuously to a depth of 7.5-ft bgs, at 2.5-ft intervals to depths between 20 and 40 ft bgs and at 5.0-ft intervals thereafter. Each boring type was sampled using an 18-inch split spoon sampler (AASHTO T-206 "Standard Method for Penetration Test and Split Barrel Sampling of Soils."). The soil samples obtained from the exploration program were visually observed in the field by the NEAS field representative and preserved for review by a Geologist for possible laboratory testing. Standard penetration tests (SPT) were conducted using a CME auto hammer that has been calibrated to be between 68.4% and 89% efficient as indicated on the boring logs (Appendix B).

Field boring logs were prepared by drilling personnel and included pavement description (where present), lithological description, SPT results recorded as blows per 6-inch increment of penetration and estimated unconfined shear strength values on specimens exhibiting cohesion (using a hand-penetrometer). Groundwater level observations were recorded both during and after the completion of drilling. These



groundwater level observations are included on the individual boring logs (provided in Appendix B). After completing the borings, the boreholes were backfilled with either auger cuttings, bentonite chips, or a combination of these materials and patched accordingly with cold patch asphalt and/or cement when drilling through the roadway.

3.2. Laboratory Testing Program

The laboratory testing program consisted of classification testing and moisture content. Data from the laboratory testing program were incorporated onto the boring logs (Appendix B). Soil samples are retained at the laboratory for 60 days following report submittal, after which time they will be discarded.

3.2.1. Classification Testing

Representative soil samples were selected for index property (Atterberg Limits) and gradation testing for classification purposes on approximately 37% of the samples. At each boring location utilized for roadway purposes, the upper two samples obtained below the proposed top of subgrade elevation were generally tested while additional samples in each boring were selected for testing with the intent of properly classifying the subsurface soil and groundwater conditions within the planned project limits. Soils not selected for testing were compared to laboratory tested samples/strata and classified visually. Moisture content testing was conducted on all samples. The laboratory testing was performed in general accordance with applicable AASHTO specifications and ODOT Supplements.

Final classification of soil strata in accordance with AASHTO M-145 "Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes," as modified by ODOT "Classification of Soils" was made once laboratory test results became available. The results of the soil classification are presented on the boring logs in Appendix B.

3.2.2. Standard Penetration Test Results

Standard Penetration Tests (SPT) and split-barrel (commonly known as split-spoon) sampling of soils were performed at varying intervals (i.e., continuous, 2.5-ft, or 5.0-ft intervals) in the project borings performed. To account for the high efficiency (automatic) hammers used during SPT sampling, field SPT N-values were converted based on the calibrated efficiency (energy ratio) of the specific drill rig's hammer. Field N-values were converted to an equivalent rod energy of 60% (N_{60}) for use in analysis or for correlation purposes. The resulting N_{60} values are shown on the boring logs provided in Appendix B.

4. FINDINGS

The subsurface conditions encountered during NEAS's explorations are described in the following subsections and/or on each boring log presented in Appendix B. The boring logs represent NEAS's interpretation of the subsurface conditions encountered at each boring location based on our site observations, field logs, visual review of the soil samples by NEAS's geologist, and laboratory test results. The lines designating the interfaces between various soil strata on the boring logs represent the approximate interface location; the actual transition between strata may be gradual and indistinct. The subsurface soil and groundwater characterizations included herein, including summary test data, are based on the subsurface findings from the geotechnical explorations performed by NEAS as part of the referenced project. At the time of the composition of this report, proposed grade and pavement section information has been assumed to match the Stage 2 plan set provided by AECOM dated February 8, 2023. It should be noted that for the purposes of this report and our analysis, the term 'proposed subgrade' has been assumed



to represent soils and/or soil conditions extending to a depth of 6 ft below the bottom of proposed pavement section (i.e., top of subgrade).

4.1. Subgrade Conditions

The following subsections present a brief summary of the subsurface conditions by roadway segment.

4.1.1. State Route 14

The project portion of SR-14 is currently bridge deck (existing Bridge CUY 14 0693) with the exception of the first and last approximate 250 ft which is pavement supported on grade. SR-14 is planned for full depth pavement replacement where the majority of the new pavement will be supported on greater than 3 feet of new fill. At the beginning of the project, SR-14 from about STA. 366+40 to 368+85 will be supported by existing subgrade soils as will the portion of SR-14 from about STA. 382+00 to 385+40.

Along SR-14 from about STA. 366+40 to 368+85, one (1) of the samples taken along this portion of roadway were identified as non-cohesive soil and was comprised of Gravel and Stone Fragments with Sand (A-1-b) while another samples was classified as cohesive Sandy Silt (A-4a). With respect to the relative density of the non-cohesive soils, the sample can be described as loose correlating to an SPT-N value (N_{60}) of 10 blows per foot (bpf). Natural moisture contents of the non-cohesive soil sample was estimated to be 10 percent. With respect to the consistency of the cohesive soil at this location, the material is characterized as stiff correlating to an N_{60} value of 15 bpf. Natural moisture content of the cohesive material was 15 percent. Based on an Atterberg Limits test performed on a representative sample of the cohesive soil, the liquid and plastic limits were estimated to be 23 percent and 16 percent, respectively.

Along SR-14 from about STA. 382+00 to 385+40, one hundred percent (100%) of the samples taken along this portion of roadway were classified as fine-grained, cohesive soils and were comprised of Clay (A-7-6). With respect to the consistency of the cohesive soils, the descriptions varied from medium stiff to very stiff correlating to N₆₀ values ranging from 8 to 11 bpf. Natural moisture contents ranged from 22 to 31 percent. Based on Atterberg Limits test performed on representative samples of the cohesive soils, the liquid and plastic limits were estimated to range from 42 to 56 percent and from 20 to 26 percent, respectively.

4.1.2. Henry Street

The project portion of Henry St is also planned for full depth pavement replacement. Along Henry St, one hundred percent (100%) of the samples taken along this portion of roadway were classified as non-cohesive soil and were comprised of Gravel and Stone Fragments with Sand (A-1-b). With respect to the relative density of the non-cohesive soil, the samples can be described as medium dense to dense correlating to N_{60} values ranging from 22 to 46 bpf. Natural moisture contents ranged from 4 to 10 percent.

4.1.3. Chaincraft Road

The project portion of Chaincraft Rd is planned for full depth pavement replacement. Along Chaincraft Rd, eighty-five percent (85%) of the samples taken along this portion of roadway were classified as non-cohesive soil and were comprised of: 1) Gravel and/or Stone Fragments with Sand (A-1-b, 31% of samples); 2) Coarse and Fine sand (A-3a, 31% of sample); and, 3) non-cohesive Sandy Silt (A-4a, 23% of samples). With respect to the relative density of the non-cohesive soil, the descriptions varied from very loose to medium dense correlating to N₆₀ values between 1 and 27 bpf. Natural moisture contents ranged from 14 to 29 percent.

The remaining fifteen percent (15%) of the samples were identified as fine-grained, cohesive soils comprised of cohesive Sandy Silt (A-4a). With respect to the consistency of the cohesive soil, the



descriptions varied from medium stiff to stiff correlating to N_{60} values ranging from 8 to 11 bpf. Natural moisture contents ranged from 5 to 11 percent. Based on an Atterberg Limits test performed on a representative sample of the cohesive soil, the liquid and plastic limits were estimated to be 22 percent and 15 percent, respectively. Natural moisture contents ranged from 11 to 19 percent.

4.1.4. Groundwater

Groundwater measurements were taken during the boring drilling procedures and/or immediately following the completion of each borehole. Groundwater was not encountered within the subgrade depth (top 7.5 ft below proposed pavement) of the 22 project borings performed by NEAS for the referenced project with the exception of boring B-008-0-21 performed for Retaining Wall 1. In boring B-008-0-21, groundwater was encountered at a depth of 7 ft (elevation 808.0 ft amsl). Groundwater was encountered within 12 of the 22 borings performed at depths ranging from 7 to 38.2 ft bgs (802.9 ft amsl to 816.5 ft amsl).

It should be noted that groundwater is affected by many hydrologic characteristics in the area and may vary from those measured at the time of the exploration. The specific groundwater readings are included on the individual test boring logs located within Appendix B.

5. ANALYSES AND RECOMMENDATIONS

We understand that full depth pavement replacement of SR-14, Henry St, and Chaincraft Rd is planned as part of the CUY-14-6.93 project (PID 104132) to replace the existing bridge over Chaincraft Rd, Norfolk Southern Rwy and Wheeling & Lake Erie Rwy with a new shortened structure on a new alignment. It is also our understanding that the existing bridge (CUY-CR24-0062) along Henry St over culvert CUY-CR00240-00.610 will be removed and replaced with embankment fill. Therefore, full depth pavement replacement is planned where existing pavement is present while new pavement is planned atop embankment fill where existing bridges are either removed or shortened.

The following sub-sections present the analyses and recommendations for: 1) the subgrade stabilization and pavement design parameters for SR-14, Henry St, and Chaincraft Rd; and, 2) proposed embankment alterations and constriction. In general, the pavement subgrade analysis and recommendations presented are in accordance with ODOT's *Geotechnical Bulletin 1* (GB1) (ODOT [1], 2022) and *Pavement Design Manual* (PDM) (ODOT PDM, 2021).

5.1. Subgrade Analysis

A GB1 analysis was performed to identify the method, location, and dimensions (including depth) of required subgrade stabilization for the project. In addition to identifying stabilization recommendations, pavement design parameters are also determined to aid in pavement section design. The subsections below present the results of our GB1 analysis including pavement design parameters and unsuitable subgrade conditions identified within the project limits. GB1 analysis spreadsheets are provided in Appendix C.

Again, it should be noted that for the purposes of this report and our analysis, the term 'proposed subgrade' has been assumed to represent soils and/or soil conditions extending to a depth of 6 ft below the bottom of proposed pavement section (i.e., top of subgrade).

5.1.1. Pavement Design Recommendations

It is our understanding that pavement analysis and design is to be performed to determine the proposed pavement sections for the segments within the project limits to undergo full depth replacement. A GB1 analysis was performed using the subgrade soil data obtained during our field exploration program to



Cuyahoga County, Ohio

PID: 104132

evaluate the soil characteristics to develop pavement parameters for use in pavement design. The subgrade analysis parameters recommended for use in pavement design are presented in Table 1 below. Provided in the table are ranges of maximum, minimum and average N_{60L} values for the indicated segments as well as the design CBR value recommended for use in pavement design.

Table 1: Pavement Design Values

Segment	Maximum N _{60L}	Minimum N _{60L}	Average N _{60L}	Average PI Values	Design CBR
SR-14 (382+00 to 385+40) / Old Broadway Ave	10	8	9	26	4
SR-14 (366+40 to 368+85)	10	10	10	7	13
Henry St	22	22	22	-	13
Chaincraft Rd	10	1	5	7	11

5.1.2. Unsuitable Subgrade

Per ODOT's GB1, the presence of select subgrade conditions are prohibited within the subgrade zone for new pavement construction. These unsuitable subgrade conditions generally include the presence of rock, specific soil types, weak soil conditions, and overly moist soil conditions. With respect to the proposed pavement construction and widening project these subgrade conditions are further discussed in the following subsections.

5.1.2.1. Rock

Rock was not encountered at or close to subgrade elevation at the boring locations performed within the project limits. Per ODOT's GB1, if rock is encountered within 24 inches of the bottom of the proposed asphalt or concrete pavement it is to be removed in accordance with 204.05 of the ODOT CMS and replaced with Item 204 Embankment.

5.1.2.2. Prohibited Soils

Prohibited soil types per the GB1, which include A-4b, A-2-5, A-5, A-7-5, A-8a, A-8b, and soils with liquid limits greater than 65, were not encountered within the proposed subgrade limits of the referenced project roadway segments.

5.1.2.3. Weak Soils

The GB1 recommends subgrade stabilization for soils in which the N₆₀ value of a particular soil sample (SS) at a referenced boring location is less than 12 bpf and in some cases less than 15 bpf (i.e., where moisture content is greater than optimum plus 3 percent). Based on the specific N₆₀ value at the subject boring, *Figure B - Subgrade Stabilization* within the GB1 recommends a depth of subgrade stabilization for ODOT standard stabilization methods. For the purposes of this report the term 'weak soils' has been assumed to represent subgrade soils of these conditions. It should be noted that although a soil sample's N₆₀ value may meet the criteria to be considered a weak soil, the depth in which the weak soil is encountered in relation to the proposed subgrade is considered when each individual subgrade boring is analyzed. For example, if the GB1 recommends an excavate and replace of 12 inches within a weak soil underlying 18 inches of stable material, it would be unreasonable to recommend the removal of both the stable and unstable material for a total of 30 inches of excavate and replace.

Based on N₆₀ values encountered within the project borings, our GB1 analysis suggests the need for 12 to 14 inches of either chemical treatment or excavate and replace along the referenced project roadway segments. A summary of the boring locations where weak soils were encountered and determined to have a potential impact on subgrade performance are shown in Table 2 below, per the roadway segment for



SS-4

SS-1

PID: 104132

B-019-0-21

B-021-0-21

which they were encountered. Also included is the associated GB1 recommended remediation depth at that location.

Moisture Remediation Depth (inches) Depth Below Excavate and Replace **Boring ID** Sample ID N₆₀ **Excavate and Replace Chemical Stabilization** Optimum Subgrade (ft) (Item 204 w (Item 204 w/ Geogrid - SS 861 (Item 206) (%) Geotextile) Roadway Segment: SR-14 (382+00 to 385+40) / Old Broadway Ave B-018-0-21 SS-1 8 6 1.6 - 3.1 12 14 B-019-0-21 SS-2 11 7 0.0 - 0.3 12 12 B-019-0-21 SS-3 10 13 0.3 - 1.8 12 14

Table 2: Weak Soil Locations Summary

It should be noted that *Figure B - Subgrade Stabilization* does not apply to soil types A-1-a, A-1-b, A-3, or A-3a, nor to soils with N_{60L} values of 15 or more. Per GB1 guidance, *these soils should be reworked to stabilize the subgrade*.

Roadway Segment: Chaincraft Rd

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5.1.2.4. High Moisture Content Soils

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1.8 - 3.3

1.7 - 3.2

High moisture content soils are defined by the GB1 as soils that exceed the estimated optimum moisture content (per *Figure A - Optimum Moisture Content* within the GB1) for a given classification by 3 percent or more. Per the GB1, soils determined to be above the identified moisture content levels are a likely indication of the presence of an unstable subgrade and may require some form of subgrade stabilization. Similar to our analysis of weak soils, although a soil sample's moisture content may meet the criteria to be considered high, the depth in which the high moisture soil is encountered in relation to the proposed subgrade is considered when each individual subgrade boring is analyzed for stabilization recommendations. Summaries of the boring locations where high moisture content conditions were encountered within the limits of each alignment are shown in Table 3 below.

Moisture **Optimum Moisture** Depth Below **Boring ID** Content (%) Content (%) Subgrade (ft) SR-14 (382+00 to 385+40) / Old Broadway Ave B-018-0-21 18 1.6 - 3.1 B-019-0-21 30 23 0.0 - 0.3 B-019-0-21 31 18 0.3 - 1.8 B-019-0-21 28 18 1.8 - 3.3 Chaincraft Rd B-008-0-21 1.5 - 3.0 16

Table 3: High Moisture Content Soil Locations

5.2. Stabilization Recommendations

5.2.1. Subgrade Stabilization

Unstable subgrade conditions that require stabilization per GB1 guidelines were encountered within the proposed roadway subgrade within the project limits. Unstable soils (i.e., weak soils), as previously indicated in Section 5.1. of this report, were encountered within the subgrade depths in various borings performed throughout the project. Based on our analyses, as unstable soils that require stabilization per GB1 guidelines were encountered, it is our opinion that these select areas along the project roadway segments should be stabilized via localized undercut consisting of 12 inches of Excavate and Replace (Item 204) with geotextile.



14

12

PID: 104132

Actual depths and limits of undercuts should be determined in the field by the Project Engineer based on ODOT's Subgrade Compaction and Proof Rolling specifications (Item 204).

Based on: 1) the results of our GB1 analysis; 2) the review of the unstable subgrade conditions as described in Section 5.1. of this report; and, 3) the subsequent conclusions regarding recommended stabilization, Table 4 below presents our recommendations for subgrade stabilization depths for each roadway segment included within the project.

Excavate and Replace Chemical Unsuitable Start Station End Station (Item 204 w/ Geotextile) Stabilization Subgrade Remarks (inches) (inches) Conditions SR-14 (382+00 to 385+40) / Old Broadway Ave Areas of SR-14 where full depth pavement 382+00 385+40 12 14 replacement is required and less than 12 inches of N₆₀₁ new fill is proposed. SR-14 (366+40 to 368+85) 'Reworked" and prepared in accordance with typical 366+40 368+85 N_{60L} Subgrade Compaction and Proof Rolling (Item 204) procedures and specifications. Henry St 'Reworked" and prepared in accordance with typical Begin Work **End Work** Subgrade Compaction and Proof Rolling (Item 204) procedures and specifications. Chaincraft Rd 'Reworked" and prepared in accordance with typical Begin Work **End Work** Subgrade Compaction and Proof Rolling (Item 204) procedures and specifications.

Table 4: Summary of Stabilization

Subgrade stabilization is estimated to extend to the depths indicated within Table 4 with any excavated material being replaced with material in accordance with Section F "Excavate and Replace (Item 204)" of the ODOT GB1. Stabilization limits should extend 18-inches beyond the edge of the proposed paved roadway, shoulder or median.

5.3. Embankment Stability Analysis

For purposes of evaluating stability of the planned roadway embankments proposed as part of the project, NEAS reviewed cross-sections along the length of the project roadway segments to identify sections that were interpreted to represent conditions that posed the greatest potential for slope instability as a result of the planned construction. In general, cross-sections along each of the proposed roadway alignments were reviewed to identify planned alterations that may present a combination of existing subsurface conditions and planned site grading (i.e., cutting and/or filling) that would potentially be critical to the stability of the existing and/or proposed slopes at the site. Based on our review of the available information along the referenced alignments and the associated soil properties, two cross-sections were estimated to be most "critical" for the project and were analyzed for global stability. The cross-sections selected to be evaluated are the proposed cross-sections at approximate STA. 372+50 along the SR-14 alignment and STA. 18+50 along the Henry St alignment.

For the cross-sections, NEAS developed a representative cross-sectional model to use as the basis for global stability analyses. The model was developed from NEAS's interpretation of the available information which included: 1) the project's Stage 2 plan set dated February 8, 2023 provided by AECOM; 2) a live load surcharge of 250 pounds per square foot (psf) to account for traffic induced loads; and, 3) test borings and laboratory data developed as part of this report.



For analysis purposes, borings performed along or adjacent to the indicated proposed embankment section were reviewed and a generalized material profile was developed for analysis to represent worse case conditions at the cross-section location. Utilizing the generalized soil profile, engineering properties for each soil strata were estimated based on the field (i.e., SPT N₆₀ Values, hand penetrometer values, etc.) and laboratory (i.e., Atterberg Limits, grain size, etc.) test results using correlations provided in published engineering manuals, research reports and guidance documents.

The referenced slope stability model was analyzed for long-term (Effective Stress) and short-term (Total Stress) slope stability utilizing the software entitled *Slide2* by Rocscience, Inc. Specifically, the Modified Bishop and Spencer analysis methods were used to calculate a factor of safety (FOS) for circular and block type slope failures, respectively. The FOS is the ratio of the resisting forces and the driving forces, with the desired safety factor being more than about 1.3 which equates to an AASHTO resistance factor less than 0.75 (per AASHTO's LRFD BDS the specified resistance factors are essentially the inverse of the FOS that should be targeted in slope stability programs). For this analysis, a resistance factor of 0.75 or lower is targeted as the slope does not contain or support a structural element.

Based on our slope stability analyses for the above referenced roadway embankment sections, the minimum slope stability safety factor is about 1.3 (0.75 resistance factor) for the section analyzed along SR-14 near STA. 372+50. The graphical output of the slope stability program (cross-sectional model, calculated safety factor, and critical failure plane) for the analyzed section is presented in Appendix D.

5.4. Embankment Construction Recommendations

As indicated above, the embankment cross-section analyzed for slope stability was determined to be stable (i.e., FOS greater than about 1.3) as proposed in the project's Stage 2 plans dated February 8, 2023 provided by AECOM. Therefore, the proposed embankment slopes can be constructed in accordance with Item 203 "Roadway Excavation and Embankment" of the ODOT CMS.

In areas where additional embankment material is proposed along existing slopes that are steeper than 8 Horizontal to 1 Vertical (8H:1V) but flatter than 4H:1V, it is recommended that the proposed embankment be benched into the existing slopes in accordance with Item 203.05 "Embankment Construction Methods" of the ODOT CMS. For areas where additional embankment material is proposed along existing slopes that are steeper than 4H:1V, it is recommended that the proposed embankment be designed and constructed in accordance with GB2. For sidehill fills planned on existing slopes steeper than 4H:1V, ODOT's GB2 recommends that the embankment slopes be constructed utilizing special benching in order to blend the new embankment with the existing slope to prevent the development of a weak shear plane at the interface between the proposed fill and existing slope material (ODOT [2], 2017). As the project embankment fill slopes were determined to be stable as-proposed based on our embankment stability analysis, a special benching scheme similar to that shown in Figure 1 of the ODOT GB2 can be used in areas where special benching is recommended. The height and width dimensions of the special benching scheme shown in Figure 1 should be arranged to minimize the required cut and fill quantities, though the height of a single bench shall not exceed 20 ft without a stability analysis and design per OSHA requirements. Additionally, it may be appropriate to adjust the bench slope shown from a 1H:1V to a 1.75H:1V slope if the existing slope is made up of primarily granular materials. The benched material should be replaced with compacted engineered fill per Item 203 of the ODOT CMS, while proper lift thicknesses and material density should be maintained in the proposed fill per Item 203.06 of the ODOT CMS. In situations where it is not practical to extend the final bench through the existing roadway due to maintenance of traffic concerns, a benching scheme similar to that shown in Figure 1a of the ODOT GB2 can be used in order to avoid impacting the existing roadway, guardrail or shoulder. This scheme results in the placement of a temporary oversteepened fill that can later be "shaved-off" to bring the slope to the final proposed grade.



PID: 104132

QUALIFICATIONS

This investigation was performed in accordance with accepted geotechnical engineering practice for the purpose of characterizing the subsurface and groundwater conditions within the project limits. This report has been prepared for AECOM, ODOT and their design consultants to be used solely in evaluating the roadway subgrade soils within the project limits and presenting geotechnical engineering recommendations specific to this project. The assessment of general site environmental conditions or the presence of pollutants in the soil, rock and groundwater of the site was beyond the scope of this geotechnical exploration. Our recommendations are based on the results of our field explorations, laboratory tests results from representative soil samples, and geotechnical engineering analyses. The results of the field explorations and laboratory tests, which form the basis of our recommendations, are presented in the appendices as noted. This report does not reflect any variations that may occur between the borings or elsewhere on the site, or variations whose nature and extent may not become evident until a later stage of construction. In the event that any changes occur in the nature, design or location of the proposed roadway or structure foundation work, the conclusions and recommendations contained in this report should not be considered valid until they are reviewed, and have been modified or verified in writing by a geotechnical engineer.

It has been a pleasure to be of service to AECOM in performing this geotechnical exploration for the CUY-14-6.93 project. Please call if there are any questions, or if we can be of further service.

Respectfully Submitted,

Brendan P. Andrews, P.E. Project Manager/Sr. Geotechnical Engineer Kevin C. Arens, P.E. Geotechnical Engineer

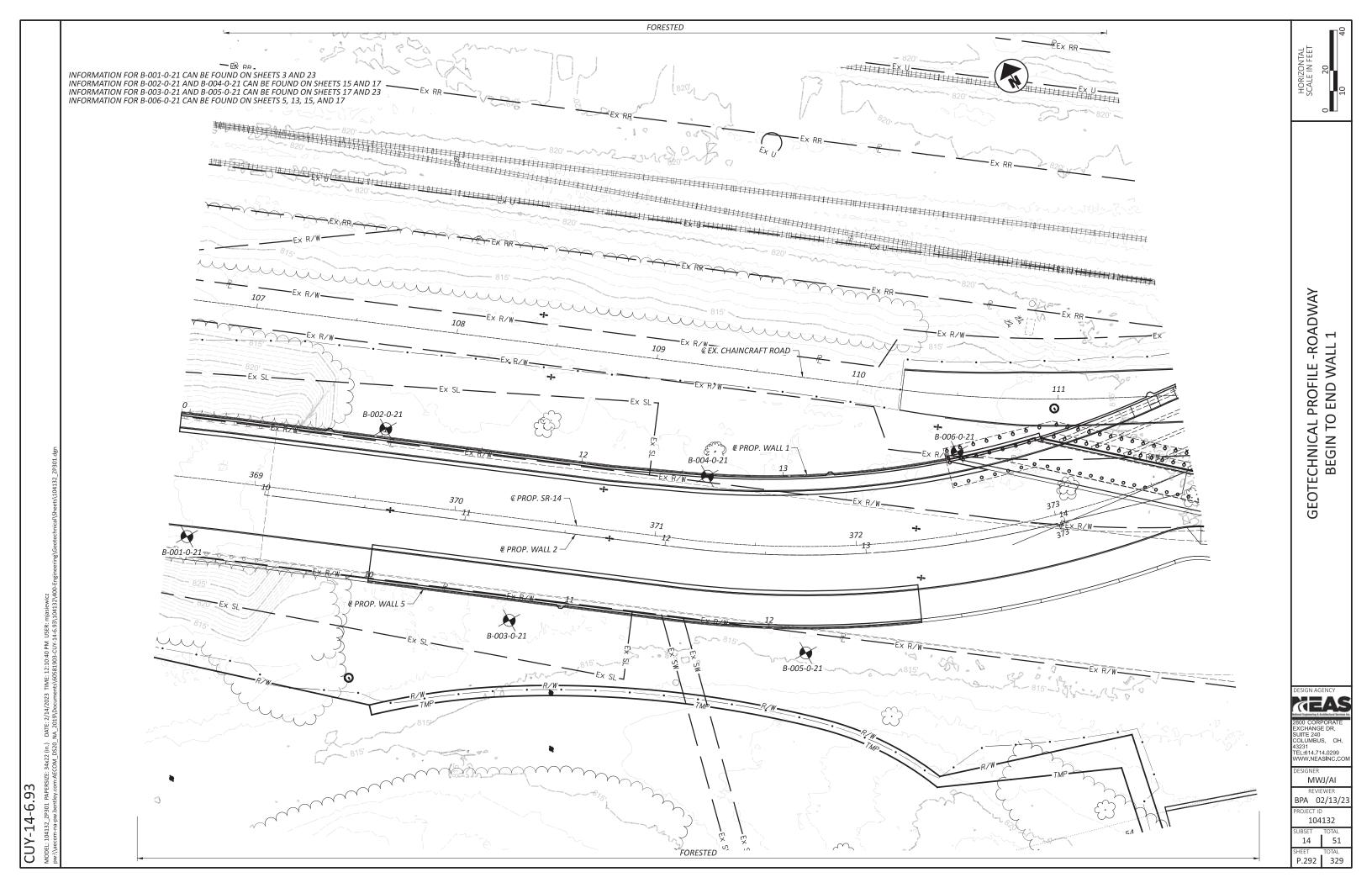


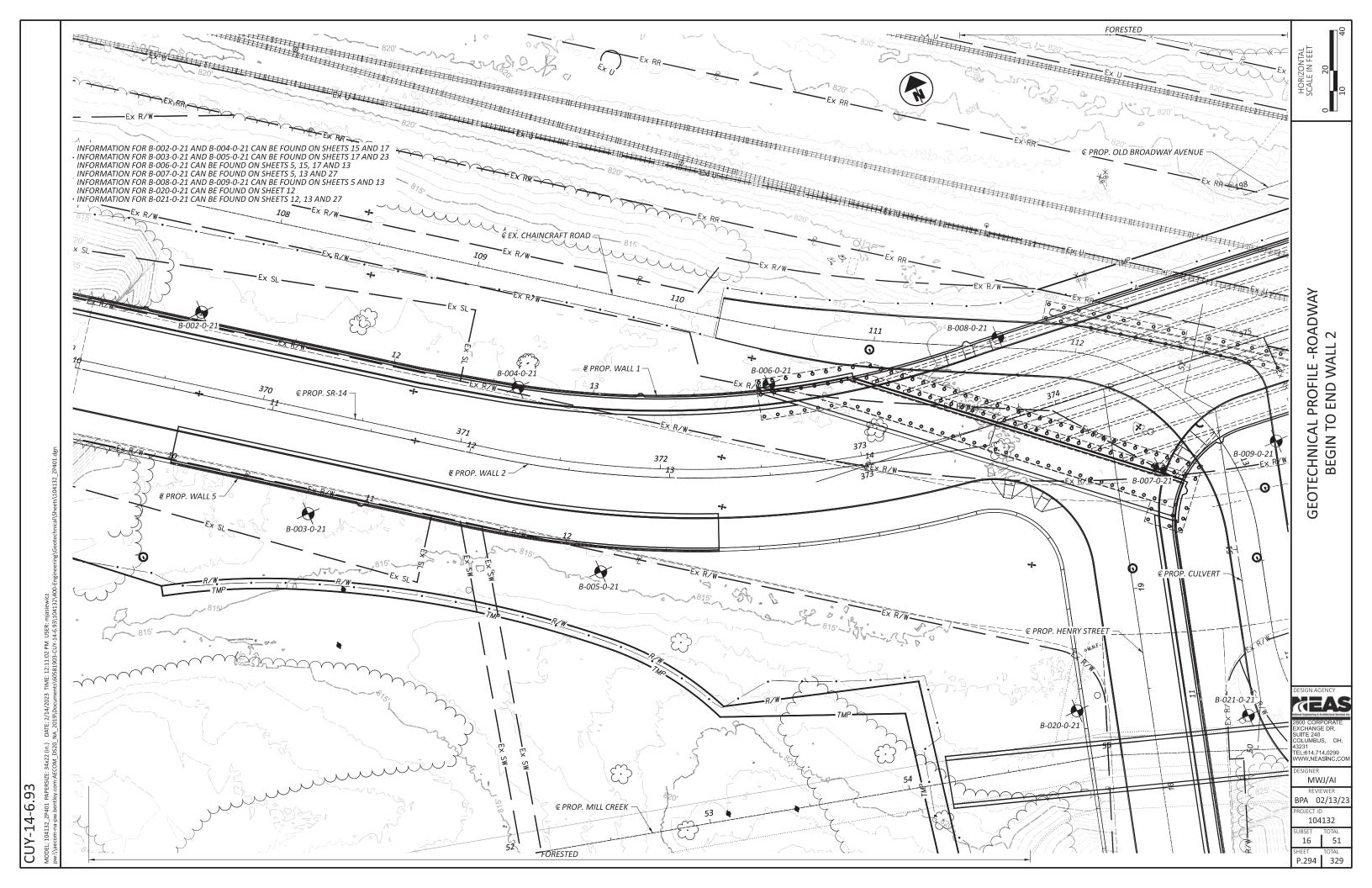
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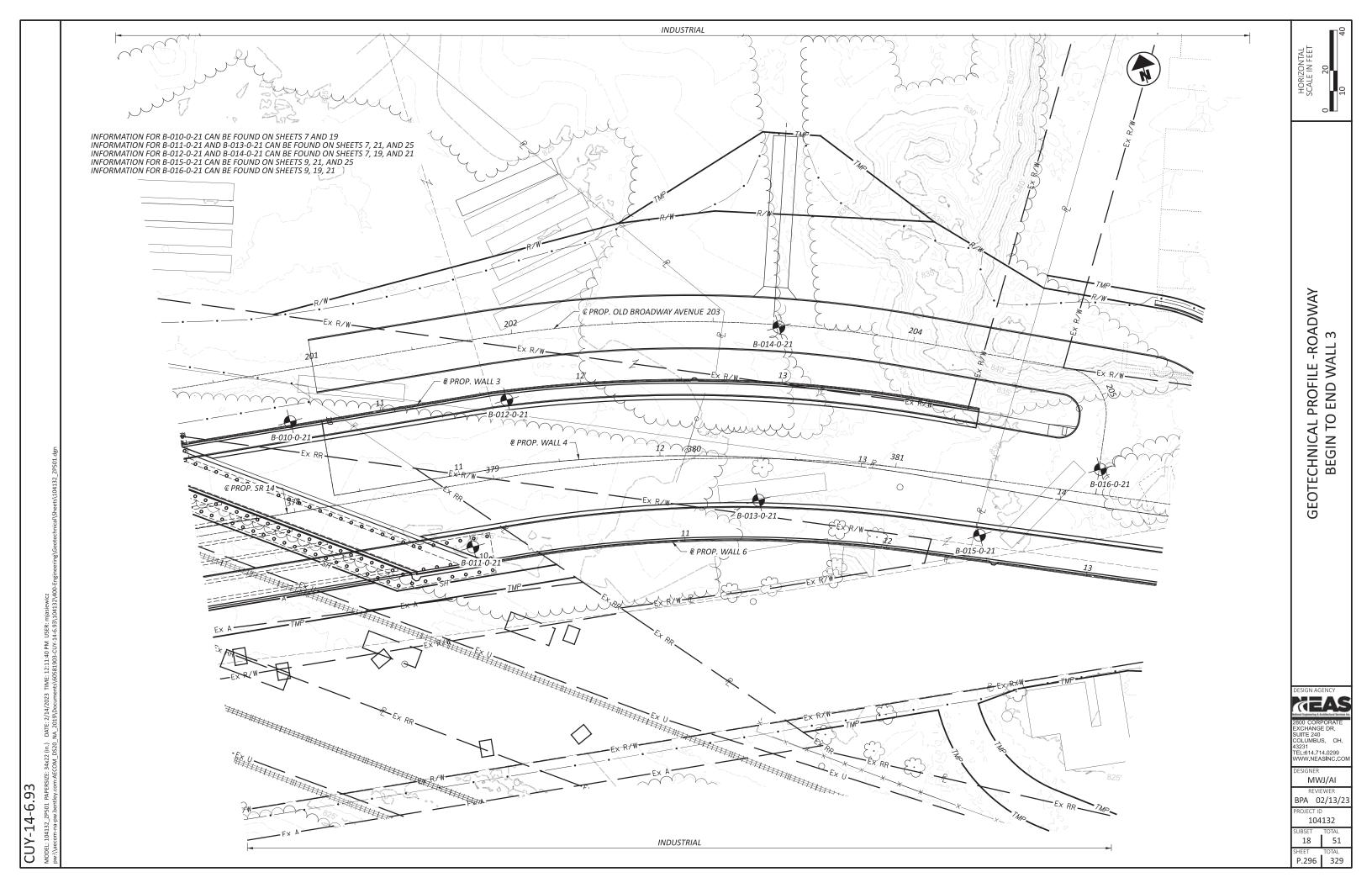
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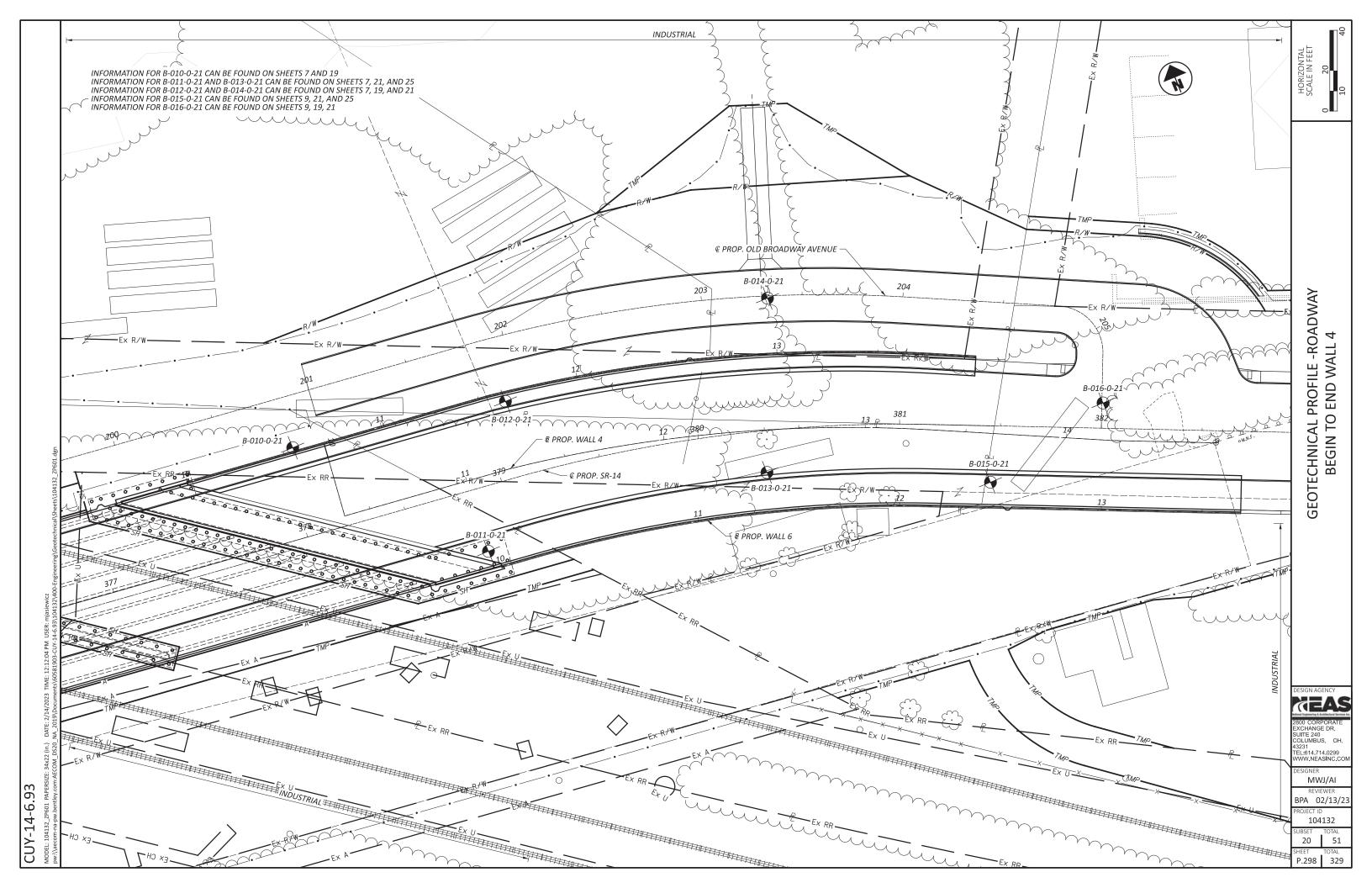


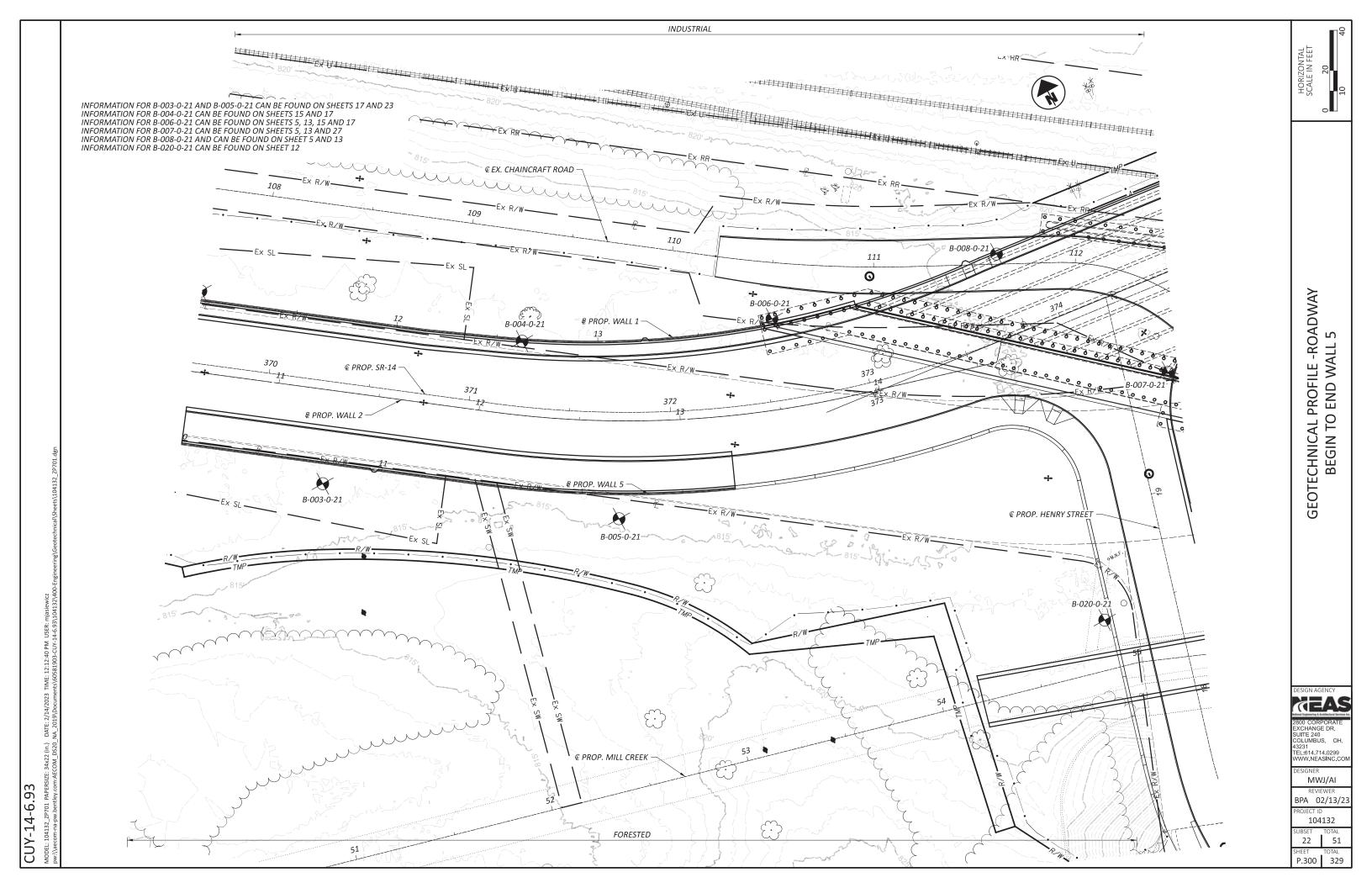
APPENDIX A SOIL PROFILE SHEETS

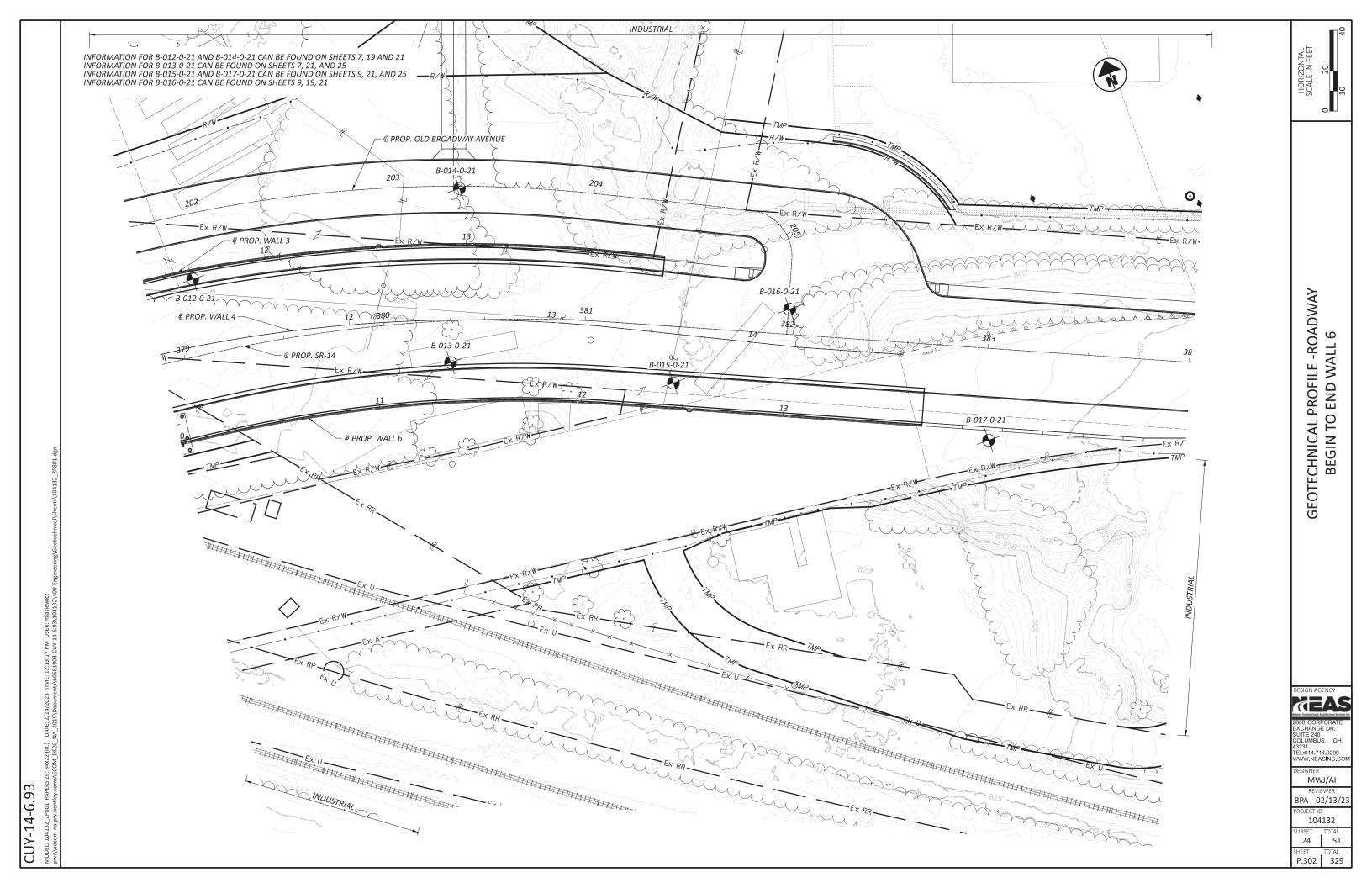


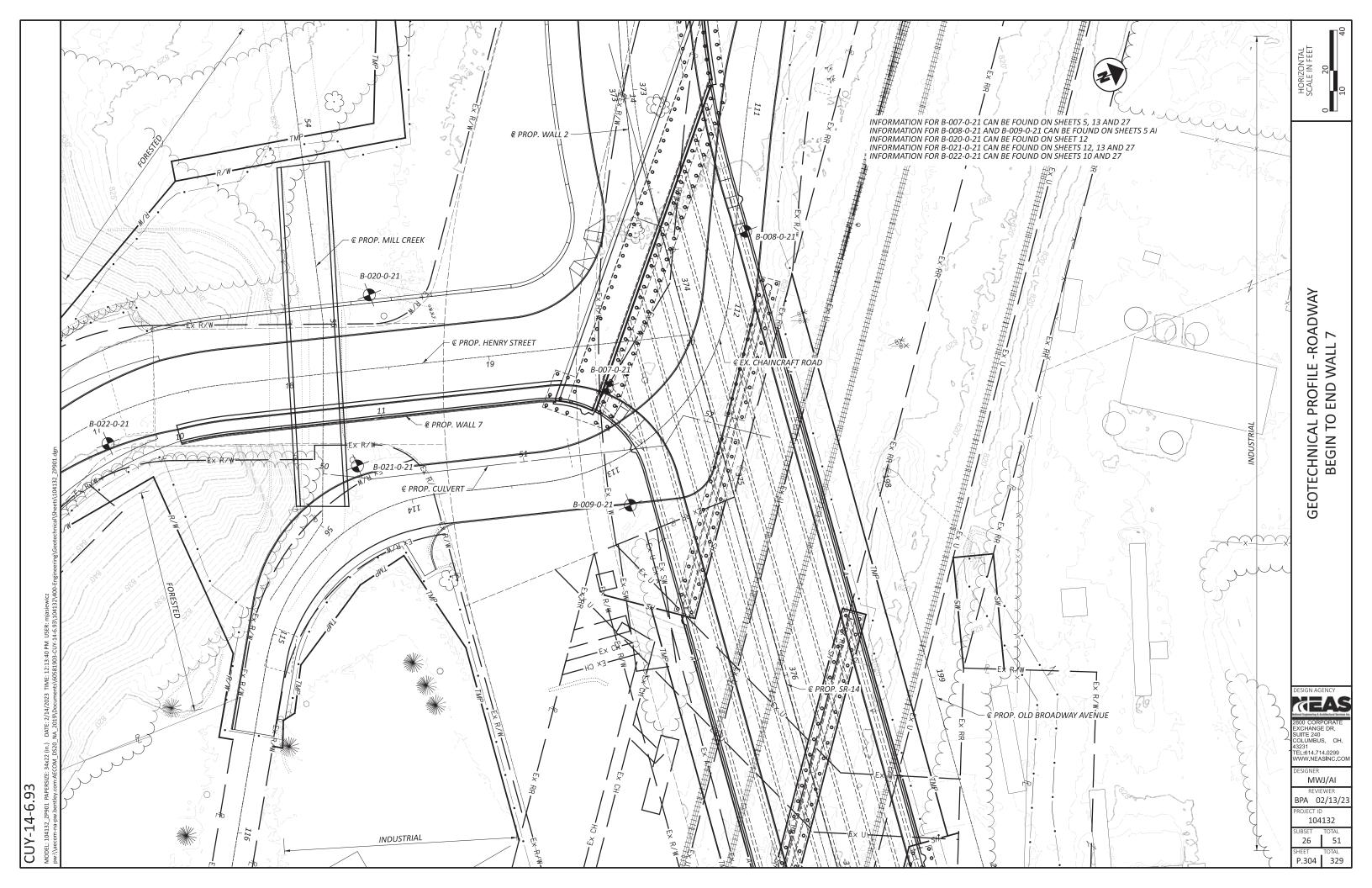












APPENDIX B BORING LOGS

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / ASHBAUGH DRILL RIG: CME 55T STATION / OFFSET: 368+69, 29' RT. B-001-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / ASHBAUGH HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 831.3 (MSL) EOB: 61.5 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/25/21 END: 8/25/21 **ENERGY RATIO (%):** 68.4 LAT / LONG: 41.431969, -81.602477 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 831.3 7.0" ASPHALT AND 14.5" BASE (DRILLERS DESCRIPTION) 829.5 2 LOOSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, CONTAINS SLAG, DAMP 3 10 100 SS-1 A-1-b (V) (FILL) 4 10 827.4 HARD, BROWN, SANDY SILT, SOME CLAY, TRACE GRAVEL, DAMP 78 SS-2 4.25 21 23 15 4 8 21 46 16 A-4a (6) 15 6 824.3 STIFF TO VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP TO MOIST 8 13 67 SS-3 3.50 A-6a (V) 6 17 9 10 13 67 SS-4 2.75 7 8 14 40 31 31 20 A-6a (8) 5 11 17 12 13 11 56 SS-5 2.75 18 A-6a (V) 15 8 44 SS-6 2.25 A-6a (V) 3 16 18 9 67 SS-7 2.25 6 10 17 38 29 30 19 11 17 A-6a (7) 19 20 5 11 78 SS-8 2.00 20 A-6a (V) 21 22 @22.5' TO 26.5': BECOMES GRAY MOTTLED WITH GRAY 17 5 42 20 8 78 SS-9 3.00 3 16 34 35 15 A-6a (10) AND ORANGISH BROWN, CONTAINS IRON STAINING 24 25 8 18 56 SS-10 2.50 22 A-6a (V) 26 804.3 27 DENSE, DARK GRAY, SANDY SILT, LITTLE GRAVEL, TRACE CLAY, CONTAINS TRACE ROOTS AND HIGHLY WEATHERED 28 31 56 9 33 8 NΡ NP 12 SS-11 19 31 NP 24 A-4a (1) WOOD FRAGMENTS, SLIGHTLY ORGANIC, WET 15 29 801.8

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EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 369+61, 36' LT. B-002-0-21 SR-14 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: PAGE 40.0 ft. PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 813.0 (MSL) EOB: 1 OF 2 SAMPLING METHOD: SPT **ENERGY RATIO (%):** LAT / LONG: 41.431962, -81.602068 START: 8/5/21 END: 8/5/21 89 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 813.0 VERY LOOSE, BROWN, COARSE AND FINE SAND, SOME SILT, TRACE TO LITTLE GRAVEL, TRACE CLAY, CONTAINS ROOTS AND TRACE IRON STAINING, WET 2 3 3 100 SS-1 A-3a (V) 1 18 5 SS-2 100 28 A-3a (V) 6 806.0 MEDIUM DENSE. BROWN. GRAVEL WITH SAND AND SILT. TRACE CLAY, CONTAINS ROOTS AND TRACE IRON 8 5 13 100 SS-3 30 14 29 21 6 NP NP NP 17 A-2-4 (0) STAINING, MOIST 9 803.5 LOOSE TO DENSE, GRAY, GRAVEL AND STONE 10 FRAGMENTS WITH SAND. TRACE TO LITTLE SILT. TRACE 2 10 100 SS-4 20 A-1-b (V) CLAY, MOIST TO WET 12 13 31 9 100 SS-5 A-1-b (V) 14 15 10 27 100 SS-6 27 37 26 8 2 NP NP NP 13 A-1-b (0) 8 16 10 17 18 11 36 39 SS-7 13 A-1-b (V) 19 20 46 11 28 SS-8 9 A-1-b (V) 21 22 5 @22.5': SS-9 BECOMES VERY DENSE 23 53 A-1-b (V) 16 50 SS-9 20 24 1 LV 1 25 58 SS-10 38 39 15 6 2 NP NP NP A-1-b (0) 19 56 10 26 20 27 28 1>11> 29

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EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 370+98, 14' RT. B-003-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 813.2 (MSL) EOB: 45.0 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/2/21 END: 8/2/21 **ENERGY RATIO (%):** LAT / LONG: 41.431652, -81.602046 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 813.2 VERY LOOSE TO MEDIUM DENSE, BROWN, COARSE AND FINE SAND, SOME SILT, LITTLE TO SOME GRAVEL, TRACE CLAY, MOIST TO WET 2 3 12 SS-1 83 A-3a (V) 4 11 2 67 SS-2 A-3a (V) 18 6 8 3 78 SS-3 17 34 23 5 NP NP NP 17 21 A-3a (0) 9 803.7 DENSE TO VERY DENSE, BROWN AND GRAY, GRAVEL AND STONE FRAGMENTS. LITTLE TO SOME SAND. TRACE 000 **W** 802.2 70 22 SS-4 14 A-1-a (V) SILT, TRACE CLAY, WET TO MOIST @11.0'; ENCOUNTERED POSSIBLE COBBLE 12 000 13 10 31 56 SS-5 12 A-1-a (V) 11 14 15 18 42 89 SS-6 A-1-a (V) 12 16 16 d 796.2 17 DENSE, BROWN AND MAROONISH GRAY, GRAVEL AND STONE FRAGMENTS WITH SAND AND SILT. LITTLE CLAY. 18 11 31 89 SS-7 32 15 18 19 16 21 13 8 12 A-2-4 (0) DAMP 794.3 10 19 DENSE. BROWN AND MAROONISH GRAY. SANDY SILT. LITTLE CLAY, TRACE TO LITTLE GRAVEL, DAMP 20 46 NP NP 12 100 SS-8 17 10 30 28 15 NP 12 A-4a (2) 21 22 1 > 1 13 NΡ 42 26 39 NP NP 100 SS-9 14 17 A-4a (4) 14 24 788.7 1>1 STIFF, MAROONISH GRAY, CLAY, SOME SILT, TRACE A-7-6 (V) 7 LV 25 787.7 21 SS-10A 1.50 10 SAND. TRACE GRAVEL. MOIST 37 100 \@25.0'; ENCOUNTERED HEAVE SS-10B 26 18 A-3a (V) 16 DENSE, GRAY, COARSE AND FINE SAND, TRACE GRAVEL, 1>11> 27 TRACE SILT, TRACE CLAY, WET 784.9 28 1>11> DENSE TO VERY DENSE, GRAY, GRAVEL WITH SAND. 29 TRACE SILT, TRACE CLAY, MOIST

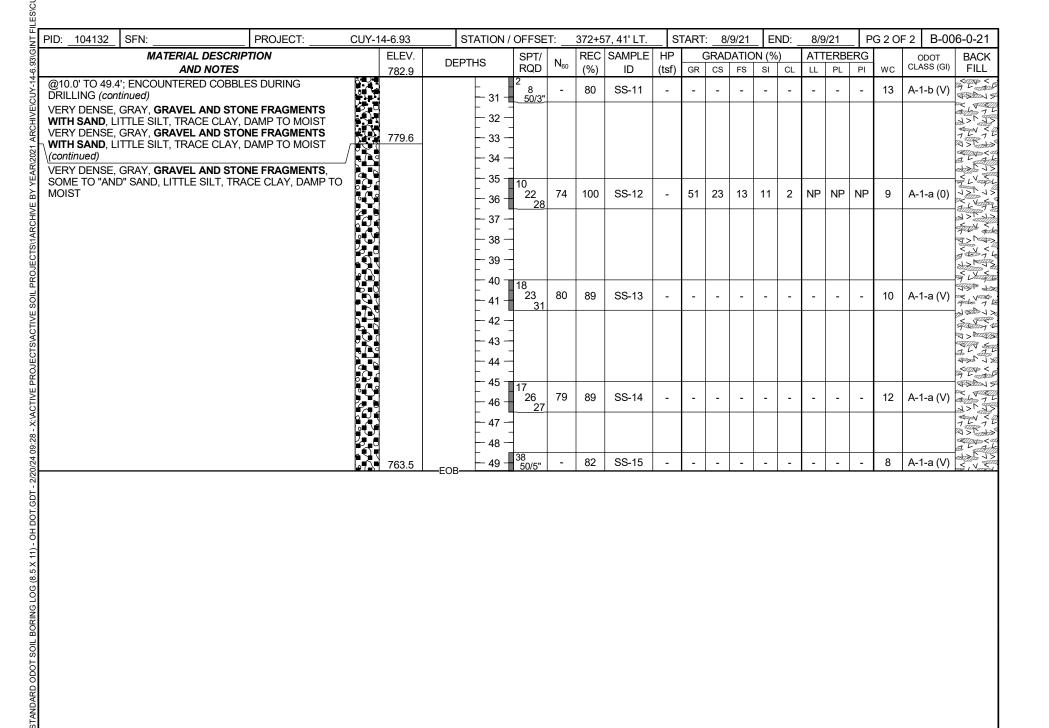
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EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 371+23, 32' LT. B-004-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 812.9 (MSL) EOB: 45.0 ft. 1 OF 2 SAMPLING METHOD: SPT **ENERGY RATIO (%):** LAT / LONG: 41.431688, -81.601606 START: 8/5/21 END: 8/5/21 89 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 812.9 VERY LOOSE TO LOOSE. BROWN. SANDY SILT. LITTLE GRAVEL, TRACE CLAY, MOIST 2 3 3 SS-1 9 36 30 9 NP NP NP 78 16 17 A-4a (1) 1 2 9 67 SS-2 A-4a (V) 16 6 805.9 LOOSE, BROWN, GRAVEL WITH SAND AND SILT, TRACE CLAY, MOIST 8 2 6 33 SS-3 32 17 30 17 4 NP NP NP 14 A-2-4 (0) 9 803.4 **w** 802.9 MEDIUM DENSE TO VERY DENSE, GRAY, GRAVEL AND 10 STONE FRAGMENTS WITH SAND. TRACE SILT. TRACE 31 SS-4 56 16 A-1-b (V) CLAY, WET TO MOIST 12 13 9 34 100 SS-5 A-1-b (V) 14 15 28 100 SS-6 29 43 18 8 2 NP NP NP A-1-b (0) 9 16 10 18 12 36 44 SS-7 A-1-b (V) 14 19 20 47 13 44 SS-8 9 A-1-b (V) 21 22 13 23 47 44 SS-9 A-1-b (V) 18 24 25 @25.0'; DIFFICULT DRILLING DUE TO POSSIBLE 12 SS-10 A-1-b (V) 16 55 33 STANDARD ODOT SOIL **ENCOUNTER WITH COBBLES AND/OR BOULDERS** 11 26 27 28 29

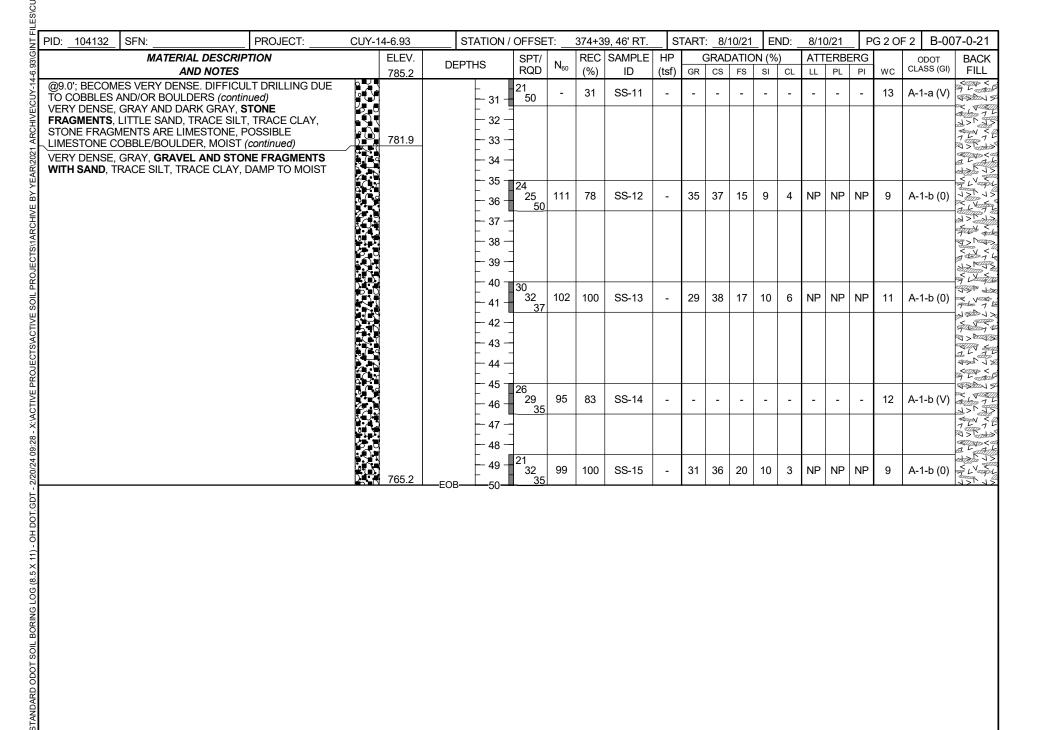
EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 372+69, 18' RT. B-005-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 813.6 (MSL) EOB: 44.8 ft. 1 OF 2 SAMPLING METHOD: SPT / ST **ENERGY RATIO (%):** 41.431413, -81.601611 START: 8/3/21 END: 8/4/21 LAT / LONG: **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ HOLE ODOT **DEPTHS** N_{60} CLASS (GI) RQD SEALED (%) GR CS FS SI CL LL PL PΙ WC AND NOTES ID (tsf) 813.6 MEDIUM DENSE. BROWN. GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, SS-1 CONTAINS A 1.5" LIMESTONE FRAGMENT, DAMP 2 3 16 SS-1 44 9 A-1-b (V) 6 5 75 ST-2 6 A-1-b (V) 806.6 VERY LOOSE TO MEDIUM DENSE, BROWN, GRAVEL WITH SAND AND SILT, LITTLE CLAY, DAMP TO MOIST 8 12 100 SS-3 29 13 28 19 11 NP NP NP 12 A-2-4 (0) 4 9 **w** 803.1 17 SS-4 18 A-2-4 (V) 801.6 12 MEDIUM DENSE TO DENSE, GRAY, FINE SAND, "AND" COARSE SAND, TRACE SILT, TRACE GRAVEL, TRACE CLAY, 13 40 13 67 SS-5 17 A-3 (V) MOIST TO WET 14 14 15 16 100 SS-6 3 43 44 8 2 NP NP NP 21 A-3 (0) 16 17 @17.5'; ENCOUNTERED HEAVE. PULLED AUGERS AND 18 12 37 100 SS-7 12 A-3 (V) REDRILLED WITH A SLURRY MIX TO COMBAT HEAVE. 13 19 794.1 DENSE TO VERY DENSE, GRAY, GRAVEL AND STONE 20 FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, 47 15 56 SS-8 12 A-1-b (V) MOIST 21 22 53 16 100 SS-9 A-1-b (V) 20 24 789.1 STIFF, GRAY, SANDY SILT, "AND" CLAY, TRACE GRAVEL, 25 13 CONTAINS A 2.0" SILT SEAM, MOIST SS-10 1.75 6 12 38 36 26 17 33 39 8 9 21 A-4a (8) 26 27 785.3 28 VERY DENSE, GRAY, GRAVEL WITH SAND, LITTLE SILT. 29 TRACE CLAY, DAMP TO MOIST

E F	PID: 104132	SFN:	PROJECT:	CUY-1	4-6.93	ST	ATION /	OFFSE	T:;	372+6	9, 18' RT.	S	TART	: 8/	3/21	EN	ID: _	8/4	/21	_ P(G 2 OF	2 B-00	5-0-21		
-6.93\GI		MATERIAL DESCRIF AND NOTES	PTION		ELEV. 783.6	DEPTHS		I DEPTI		SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)		GRAD cs	ATIO FS		CL	ATT LL	ERBE	RG PI	wc	ODOT CLASS (GI)	HOLE SEALED
:\CUY-14	TRACE CLAY,	GRAY, GRAVEL WITH SAN DAMP TO MOIST (continued '; DIFFICULT DRILLING DU	d)				- - 31 -	11 23 25	71	100	SS-11	-	-	-	-	-	-	-	-	-	12	A-1-b (V)			
ARCHIVE\CUY-14	AND/OR BOUL		JE 10 OOBBEE	(C)			- 32 - 33 -																		
				5 0 7 7 7 8			- 34 -																		
SOIL PROJECTS/TARCHIVE BY YEAR/2021							- 35 - - 36	22 34 42	113	100	SS-12	-	34	34	16	12	4	NP	NP	NP	9	A-1-b (0)			
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JEC IS/I							- 38 - - 39 -																		
טור דוט				0 V			- 40 - 41	12 16	61	100	SS-13	-	_	-	_	_	_	_	-	_	10	A-1-b (V)			
≅I	VEDV DENSE	GRAY, STONE FRAGMENT	FE WITH SAND		771.3		_ 42 _	25																	
JECISMC	TRACE SILT, 1	RACE CLAY, STONE FRAGILENT POSSIBLE LIMESTONE COL	SMENTS ARE				43 44 -	33 47		44	SS-14		_	_						_	10	A-1-b (V)			
/E PRU					768.8	—ЕОВ—	-	50/4"	_	44	33-14	-	_	-	-	-	-	_	-		10	Α- 1-D (V)			

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 372+57, 41' LT. B-006-0-21 TYPE: **BRIDGE** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 812.9 (MSL) EOB: 1 OF 2 SAMPLING METHOD: SPT START: 8/9/21 END: 8/9/21 **ENERGY RATIO (%):** LAT / LONG: 41.431548, -81.601193 ELEV. HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 812.9 4 L 100 LOOSE TO MEDIUM DENSE, BROWN, COARSE AND FINE SAND. SOME SILT. LITTLE GRAVEL. TRACE CLAY. 98800 S CONTAINS ROOTS. MOIST TO WET 2 3 SS-1 12 22 A-3a (V) 5 14 5 6 2 44 SS-2 19 A-3a (V) 6 805.9 MEDIUM DENSE TO VERY DENSE, BROWN AND GRAY. 4 > 1 mm **GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE** 8 13 39 SS-3 14 24 16 2 NP NP NP A-1-b (0) 44 14 4 SILT, TRACE CLAY, CONTAINS 1.5" STONE FRAGMENTS, 9 WET TO MOIST **₩** 802.9 S Walter @10.0' TO 49.4'; ENCOUNTERED COBBLES DURING TOTAL WAY 76 100 SS-4 A-1-b (V) 11 DRILLING NOD) 12 2 > 2008 13 73 17 NP NP 100 SS-5 21 45 14 3 NP 11 A-1-b (0) 14 798.4 1 L 100 DENSE, GRAY, COARSE AND FINE SAND, LITTLE SILT, 15 933000 S 19 TRACE GRAVEL. TRACE CLAY, WET 2× 47/8// 36 50 SS-6 18 A-3a (V) 10 16 795.9 17 DENSE TO VERY DENSE, GRAY, GRAVEL AND STONE FRAGMENTS WITH SAND. TRACE SILT. TRACE CLAY, WET 18 17 49 78 SS-7 A-1-b (V) 16 GDT TO MOIST 16 19 20 56 NP NP 16 100 SS-8 30 38 20 9 3 NP 11 A-1-b (0) 21 790.9 22 VERY DENSE. GRAY. COARSE AND FINE SAND. LITTLE N N GRAVEL, TRACE SILT, TRACE CLAY, MOIST 55 17 89 SS-9 A-3a (V) 20 S Wash 24 788.4 400 A 25 33 10 15 NP NP NP 83 SS-10 58 14 3 10 A-1-b (0) JOD 1 50 26 5000 A 20 > 2000/S 27 28 29 933000 S × 47180



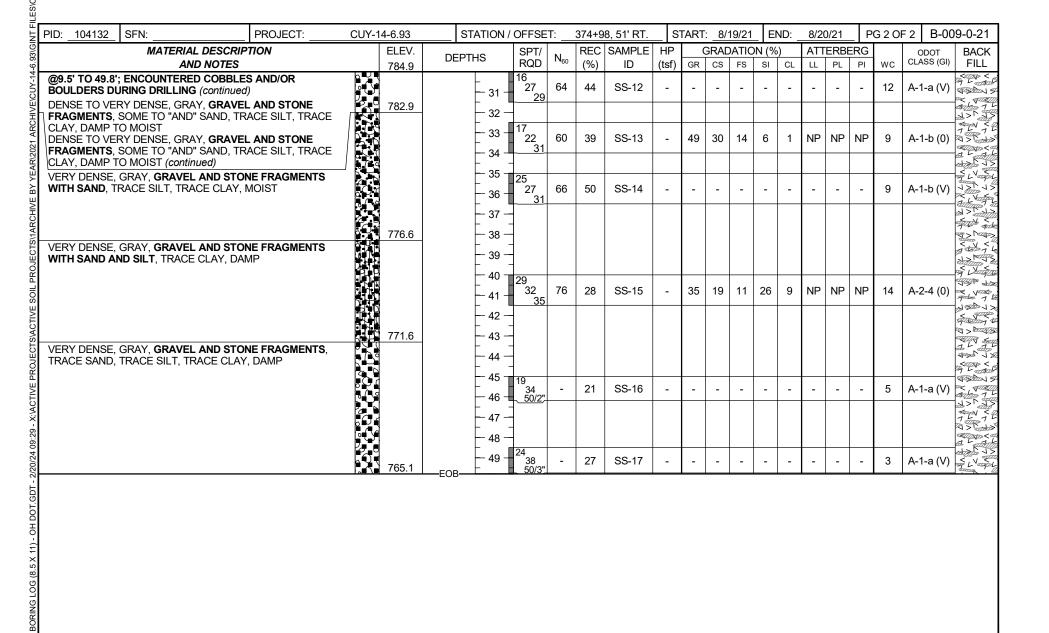
EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 374+39, 46' RT. B-007-0-21 TYPE: **BRIDGE** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14 PAGE** PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 815.2 (MSL) EOB: 50.0 ft. 1 OF 2 SAMPLING METHOD: SPT / ST **ENERGY RATIO (%):** START: 8/10/21 END: 8/10/21 89 LAT / LONG: 41.431220, -81.600616 ELEV. SPT/ SAMPLE ΗP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION REC **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 815.2 1000 C VERY LOOSE, BROWN AND GRAY BECOMING GRAY. **GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE TO** 98800 S LITTLE SILT, TRACE CLAY, WET TO DAMP SOIL PROJECTS\1ARCHIVE BY YEAR\2021 2 3 SS-1 A-1-b (V) 17 1 18 5 3 SS-2 28 A-1-b (V) 16 6 8 83 ST-3 A-1-b (V) 9 **W** 805.7 @9.0': BECOMES VERY DENSE. DIFFICULT DRILLING DUE STANDARD ODOT SOIL BORING LOG (8.5 X 11) - OH DOT.GDT - 2/20/24 09:28 - X:'ACTIVE PROJECTS'ACTIVE 74 23 23 NP NP NP 21 33 SS-4 37 13 A-1-b (0) of Varian 4 16 TO COBBLES AND/OR BOULDERS 29 300 A 11 NOD) 12 20 > \$277X/S 13 25 86 67 SS-5 10 A-1-b (V) 14 1000 < 1000 < 1000 15 933000 S 27 00 AUS 79 100 SS-6 A-1-b (V) 24 16 29 17 18 21 70 100 SS-7 12 A-1-b (V) 26 19 20 15 58 17 39 SS-8 12 A-1-b (V) 21 22 18 23 73 NP 8 NP NP 44 SS-9 32 41 15 4 10 A-1-b (0) S Wash 24 400 A 7 7 7 1 25 13 27 SS-10 86 100 A-1-b (V) 10 26 31 20 > \$277X/S 27 786.9 28 29 933000 S



EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 373+83, 41' LT. B-008-0-21 TYPE: **BRIDGE** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14 PAGE** PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA / NQ2 CALIBRATION DATE: 5/1/19 ELEVATION: 815.0 (MSL) EOB: 52.0 ft. 1 OF 2 SPT / NQ2 START: 8/18/21 END: 8/19/21 SAMPLING METHOD: **ENERGY RATIO (%):** LAT / LONG: 41.431475, -81.600782 ELEV. HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC SAMPLE HOLE ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC SEALED AND NOTES ID (tsf) 815.0 4 L 100 MEDIUM DENSE, GRAYISH BROWN, SANDY SILT, LITTLE CLAY, TRACE GRAVEL, MOIST 938Am 9 2 3 27 SS-1 32 NP NP NP 33 5 13 39 11 A-4a (2) 13 16 810.5 LOOSE TO DENSE, GRAYISH BROWN, COARSE AND FINE 5 SAND, LITTLE SILT, TRACE GRAVEL, TRACE CLAY, MOIST 22 3 10 SS-2 18 A-3a (V) TO WET 6 **W** 808.0 8 6 16 39 SS-3 A-3a (V) 9 à Naga 10 TOTAL AND 42 SS-4 A-3a (V) 44 14 JOD 1 12 10 20 > \$277X/S 13 29 12 NP NP 13 40 50 SS-5 48 4 NP 17 A-3a (0) 14 CONP < 15 933000 S 2× 47/8// 50 72 SS-6 13 A-3a (V) 14 16 20 798.0 17 VERY DENSE, GRAY, GRAVEL AND STONE FRAGMENTS WITH SAND. TRACE SILT. TRACE CLAY, MOIST TO WET 18 20 61 33 SS-7 13 A-1-b (V) @17.0' TO 52.0'; ENCOUNTERED COBBLES AND/OR STANDARD ODOT SOIL BORING LOG (8.5 X 11) - OH DOT.GDT **BOULDERS DURING DRILLING** 19 20 61 NP NP 33 SS-8 44 21 22 10 3 NP 10 A-1-b (0) 21 22 N N 23 53 28 SS-9 A-1-b (V) 19 S Wash 24 25 12 NO V 70 22 SS-10 A-1-b (V) 10 26 25 20 > \$277X/S 27 1 1 1 1 28 92 29 44 SS-11 10 A-1-b (V) 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 10 33 29 933000 S

PID	: _10)4132	SFN:	PROJECT:	CUY-1		ST	ATION /	OFFSE	T:		3, 41' LT.			: <u>8</u> /		_	ND:		9/21		G 2 O	F 2 B-0	08-0-21
			MATERIAL DESCRI	PTION		ELEV.	DEPT	HS	SPT/ RQD	N ₆₀	REC (%)				GRAD						ERG		ODOT CLASS (GI)	HOLE
VE	RY D	ENSE,	AND NOTES GRAY, GRAVEL AND STO TRACE SILT, TRACE CLAY,	ONE FRAGMENTS	\$ ()	785.0		- 04	20 29 32		50	ID SS-12	(tsf)	GR 47	20	FS 23	SI 8	CL 2	LL NP	PL NP	PI NP	wc 10	A-1-b (0)	< (ZXP) < (
(cc	ontinu		TAGE GILT, TIMOL GLAT,	WOOT TO WET	3 U 30 U			- 31 - - 32 -	32														(-)	4 A R. (1)
					9. p			_ 33 -	19 25 50/3"	-	40	SS-13	-	-	-	-	-	-	-	-	-	9	A-1-b (V)	1212
								- 34 - - 35 -																
								- 36 -	32 26 30	83	56	SS-14	-	-	-	-	-	-	-	-	-	11	A-1-b (V)	12/12/12/12/12/12/12/12/12/12/12/12/12/1
								37 -	- 00															2 > 11 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
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					6 0 2 0 1			41 -	\ <u>50/2"</u> _														(1)	
@4 CC	42.0' - DBBLI	- 52.0'; ES ANI	CORED THROUGH GRAN D BOULDERS	ITE AND SILTSTONE	9.0			- 42 - - 43 -																
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EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / JL DRILL RIG: CME 55T STATION / OFFSET: 374+98, 51' RT. B-009-0-21 TYPE: SAMPLING FIRM / LOGGER: **BRIDGE** NEAS / JL HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 814.9 (MSL) EOB: 1 OF 2 SPT START: 8/19/21 END: 8/20/21 SAMPLING METHOD: **ENERGY RATIO (%):** 68.4 LAT / LONG: 41.431188, -81.600404 ELEV. HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 814.9 1000 C LOOSE, BLACK, GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, STONE FRAGMENTS 98800 S ARE WEAK BLACK SHALE, CONTAINS SLAG, WET 2 3 8 SS-1 67 20 A-1-b (V) 4 811.1 WINDS VERY LOOSE TO MEDIUM DENSE. GRAY AND BROWN **PROJECTS/1ARCHIVE** WITH TRACE BLACK STREAKS, SANDY SILT, TRACE CLAY, 5 TRACE GRAVEL. SLIGHTLY ORGANIC, SS-3 CONTAINS A 2 NP NP 0 100 SS-2 5 49 34 10 NP 29 A-4a (2) 1.5" SANDSTONE FRAGMENT, WET 6 Par > North 8 5 11 SS-3 A-4a (V) 72 28 Salah Salah 9 805.4 **w** 804.9 S Walter DENSE TO VERY DENSE, BROWN AND GRAY, GRAVEL 10 TOTAL AND AND STONE FRAGMENTS WITH SAND. TRACE TO LITTLE 47 SS-4 A-1-b (V) 23 28 21 SILT. TRACE CLAY. WET TO DAMP 18 J 8 7 7 @9.5' TO 49.8'; ENCOUNTERED COBBLES AND/OR 12 **BOULDERS DURING DRILLING** 20 > \$277X/S 13 NP NP 16 54 100 SS-5 48 33 9 6 4 NP 12 A-1-b (0) 31 14 1 L 100 15 93000 9 ON THE 57 39 SS-6 A-1-b (V) 16 16 17 18 19 43 39 SS-7 A-1-b (V) 19 19 20 3 48 28 SS-8 11 A-1-b (V) 21 792.9 22 N N 10 23 41 28 SS-9 A-1-a (V) 20 S War 24 THE WAR April 1 25 12 NO V 20 49 28 SS-10 A-1-a (V) 11 26 20 > 2000/S 27 1 1 1 1 28 64 33 3 NP NP 26 SS-11 60 28 8 1 NP 9 A-1-a (0) 1000 C 30 29 (1800m) S



EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / JL DRILL RIG: CME 55T STATION / OFFSET: 378+05, 45' LT. B-010-0-21 SAMPLING FIRM / LOGGER: TYPE: **BRIDGE** NEAS / JL HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 821.6 (MSL) EOB: 50.0 ft. 1 OF 2 START: 8/24/21 END: 8/24/21 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 68.4 LAT / LONG: 41.431345, -81.599251 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 821.6 5.0" LIMESTONE #57 GRAVEL (DRILLERS DESCRIPTION) 1 LV 1 821.2 1>11> VERY STIFF, BROWN, SILTY CLAY, SOME SAND, SOME ~ LV ~ GRAVEL, CONTAINS BRICK FRAGMENTS, DAMP 2 シトゴミ (FILL) ~ LV ~ 3 8 13 30 39 3 44 SS-1 2.25 25 14 18 21 A-6b (5) 1>11> 18 18 817.1 1>11> VERY STIFF, BROWN, CLAY, SOME SILT, LITTLE SAND, 5 TRACE GRAVEL. MOIST 14>1 50 2.75 9 5 22 3 SS-2 10 54 48 23 25 25 A-7-6 (16) 6 814.6 1>1 1 : MEDIUM STIFF TO VERY STIFF, BROWN AND GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, CONTAINS IRON 8 2 5 78 SS-3 0.75 6 7 28 39 20 30 18 12 24 A-6a (6) STAINING, MOIST 9 ~LV 10 1>11> 2 72 SS-4 2.25 25 A-6a (V) 809.6 12 DENSE TO VERY DENSE, BROWN AND GRAY, GRAVEL, [º Qa 1>1 1 > SOME SAND, LITTLE SILT, TRACE CLAY, MOIST TO WET **₩** 808.4 13 000 32 67 SS-5 A-1-a (V) 15 14 Poor 15 1 > 1 38 56 SS-6 55 19 11 12 3 NP NP NP 10 A-1-a (0) 15 16 18 17 000 18 21 54 61 SS-7 A-1-a (V) 14 GDT 26 19 802.1 VERY DENSE, BROWN AND GRAY, COARSE AND FINE 20 19 SAND, LITTLE GRAVEL, TRACE SILT, TRACE CLAY, MOIST 59 56 SS-8 14 A-3a (V) 21 1>1 799.6 22 DENSE TO VERY DENSE. BROWN AND GRAY. GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE TO LITTLE 23 49 44 SS-9 A-1-b (V) 1< SILT. TRACE CLAY, DAMP TO WET 24 1>11> 25 1>11> 58 28 SS-10 A-1-b (V) 26 1>11> 27 28 29

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 55X STATION / OFFSET: 378+84, 31' RT. B-011-0-21 TYPE: **BRIDGE** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14 PAGE** PID: 104132 SFN: 1081085 DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 823.5 (MSL) EOB: 59.7 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/30/21 END: 8/30/21 **ENERGY RATIO (%):** 81.9 LAT / LONG: 41.431112, -81.598998 ELEV. SAMPLE HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC HOLE ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC SEALED AND NOTES ID (tsf) 823.5 < 1200 < 2700 VERY LOOSE, WHITE, SANDY SILT, TRACE CLAY, TRACE GRAVEL. UNTESTABLE MATERIAL. RESEMBLES LIME. 98800 S MOIST 2 (FILL) 121 3 0 SS-1 100 A-4a (V) 0 819.0 MEDIUM DENSE TO DENSE, GRAY, GRAVEL AND STONE 5 FRAGMENTS WITH SAND, TRACE TO LITTLE SILT, TRACE 20 28 SS-2 A-1-b (V) 11 CLAY, DAMP TO MOIST 6 8 30 22 SS-3 10 10 A-1-b (V) 12 9 S Walter 10 TOTAL AND 48 39 SS-4 18 33 31 14 NP NP NP A-1-b (0) 17 4 11 18 JOD 1 12 20 > \$277X/S @12.5': BECOMES VERY DENSE, DIFFICULT DRILLING DUE 13 20 66 22 SS-5 11 A-1-b (V) TO COBBLES AND/OR BOULDERS 1000 < 0000 < 0000 15 933000 S ON THE 57 39 SS-6 44 27 11 13 5 NP NP NP 11 A-1-b (0) 13 16 29 17 18 29 85 28 SS-7 10 A-1-b (V) 19 20 64 A-1-b (V) 30 SS-8 11 21 50/2 22 N N 5 117 22 SS-9 A-1-b (V) 49 à Naga 24 TOP AND 25 @25.0': SS-10 CONTAINS NO RECOVERY 0 SS-10 47 50/2 JOD 1 26 STANTON A 20 > \$277X/S 27 28 29 933000 S

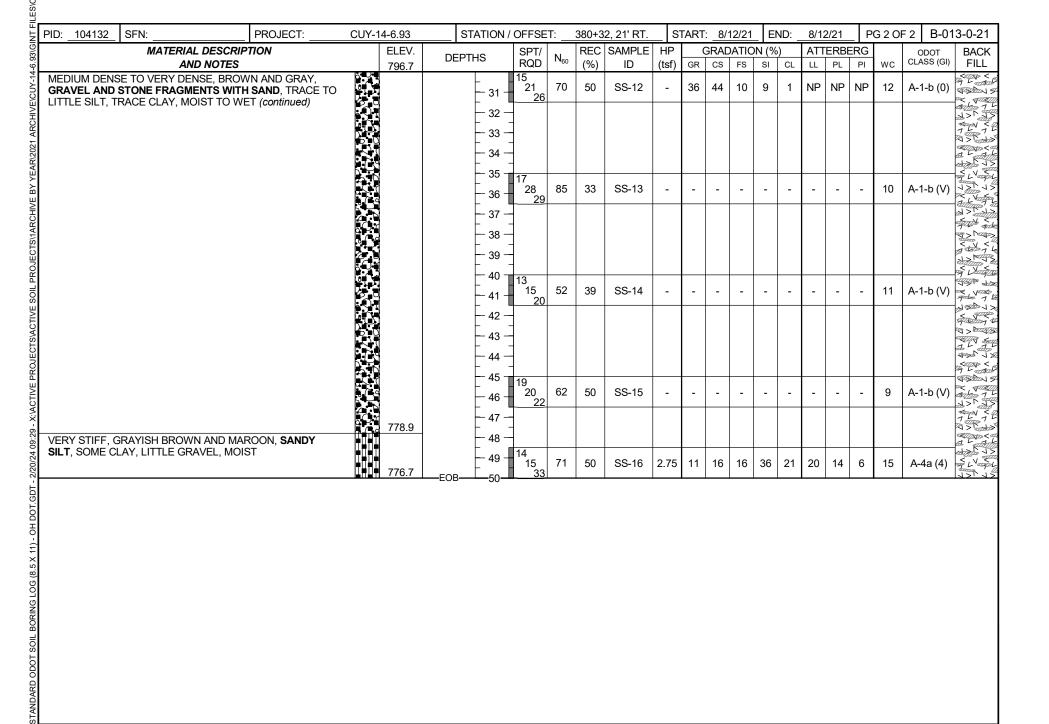
BORING LOG (8.5 X 11) - OH DOT, GDT - 2/20/24 09:29 - X: ACTIVE PROJECTS/ACTIVE SOIL PROJECTS/1ARCHIVE BY YEAR/2021 ARCHIVE/CUY

STANDARD ODOT SOIL

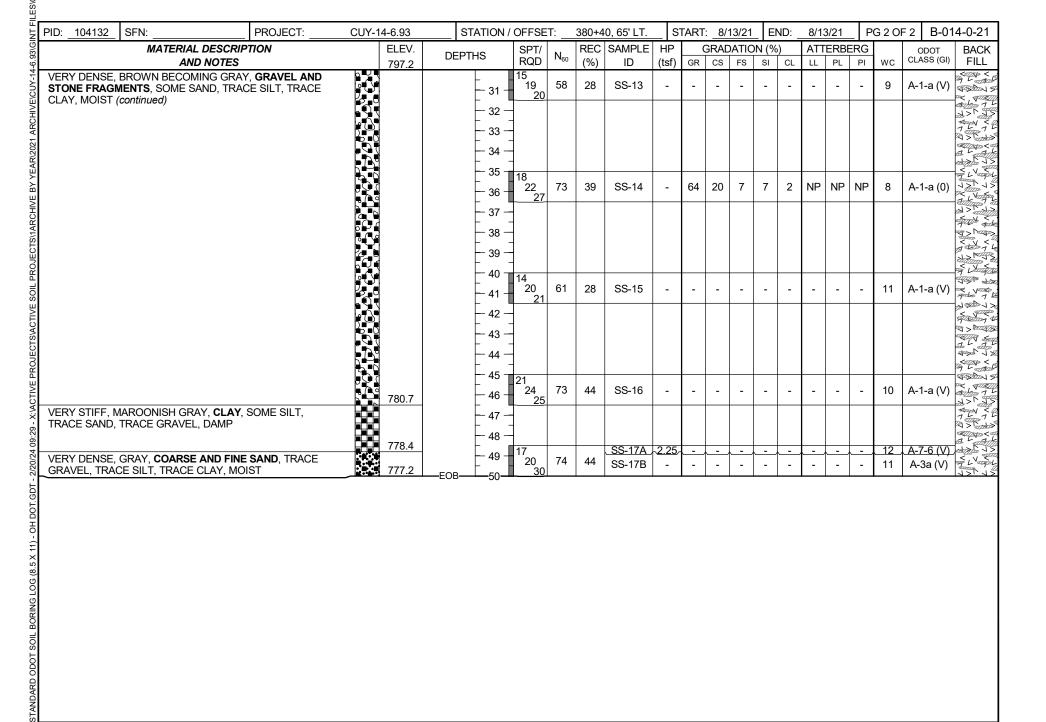
EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 379+12, 38' LT. B-012-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE ELEVATION: 821.4 (MSL) EOB: PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 50.0 ft. 1 OF 2 SAMPLING METHOD: SPT 41.431290, -81.598864 START: 8/17/21 END: 8/17/21 **ENERGY RATIO (%):** 89 LAT / LONG: **MATERIAL DESCRIPTION** ELEV. SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ REC **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 821.4 < 1200 < 2700 VERY DENSE, GRAY AND BROWN, STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, CONTAINS 2.0" 98800 S SLAG FRAGMENTS. WET TO MOIST 2 (FILL) 121 3 52 SS-1 20 12 9 NP NP NP 83 48 11 A-1-b (0) 14 11 @5.0'; CHANGES TO VERY LOOSE SS-2 17 A-1-b (V) 6 814.4 STIFF, BROWN, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL. MOIST 8 2 6 SS-3 11 1.75 20 A-6a (V) 9 811.9 w 811.4 à Valen VERY LOOSE, GRAY AND BROWN, COARSE AND FINE 10 TOTAL WAY SAND. LITTLE GRAVEL. LITTLE SILT. TRACE CLAY. WET 0 67 SS-4 25 A-3a (V) 0 NODI: 809.4 12 LOOSE, BROWN AND GRAY, GRAVEL AND STONE 2 > 2008 FRAGMENTS WITH SAND. LITTLE SILT. TRACE CLAY. WET 13 3 10 44 SS-5 16 A-1-b (V) TO DAMP @12.0' TO 50.0': ENCOUNTERED COBBLES DURING 14 CONP < DRILLING 15 933000 S @15.0' TO 29.0'; BECOMES DENSE TO VERY DENSE 2× 47/8// 45 28 SS-6 A-1-b (V) 15 16 15 18 NP 14 43 100 SS-7 46 24 11 15 4 NP NP 10 A-1-b (0) 19 20 10 40 12 56 SS-8 12 A-1-b (V) 21 22 20 A COM 52 16 39 SS-9 A-1-b (V) 19 F Wash 24 400 A 25 20 NOD 1 70 33 SS-10 A-1-b (V) 26 26 20 > \$277X/S 27 1 1 1 1 5 28 59 33 19 SS-11 A-1-b (V) 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 10 21 29 791.9 (1800m) S × 47/20/

PID:	104132	SFN:	PROJECT:	CUY-1	14-6.93	ST	ATION /	OFFSE	T:	379+1	2, 38' LT.	_ s	TART:	8/1	7/21	_ EN	ND: _	8/17	7/21	_ P	G 2 OF	2 B-0	12-0-2°
		MATERIAL DESCRI AND NOTES	PTION		ELEV. 791.4	DEPT	HS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	_	GRAD cs	ATIC FS	N (%)) CL	ATT LL	ERBE	ERG PI	wc	ODOT CLASS (GI)	BACI
ĎR	ILLING (con	D'; ENCOUNTERED COBBI			731.4		- - 31 -	9 25 26	76	28	SS-12	-	67	17	9	6	1	NP		NP	7	A-1-a (0)	
		GRAY, GRAVEL AND STO TRACE SILT, TRACE CLAY					32																7 > 0
							33 -																1 7 7
							- 34 - 35 -	14															A L
							36	11 32 35	99	56	SS-13	-	-	-	-	-	-	-	-	-	8	A-1-a (V)	77
							_ 37 _																422 × 1
		GRAY, SANDY SILT , TRA	CE GRAVEL, TRACE		783.1		- 38 - - 39 -																
CLA	AY, WET						- 40 T	19															2000 May 1900 May 190
							41	25 30	82	89	SS-14	-	7	20	36	35	2	NP	NP	NP	18	A-4a (0)	700
					778.1		42 43																\$100m
		GRAY, GRAVEL AND STO RACE SILT, TRACE CLAY			770.1		- 44 -																
							- 45 - - 46	33	98	50	SS-15	-	42	26	24	7	1	NP	NP	NP	11	A-1-b (0)	
					774.1		- 47 -	32															12/2
		GRAY, STONE FRAGMEN RACE CLAY, MOIST	ITS, TRACE SAND,				48	0.7															
					771.4	50 D	49 	37 26 30	83	44	SS-16	-	-	-	-	-	-	-	-	-	11	A-1-a (V)	A LANGE

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 380+32, 21' RT. B-013-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: ELEVATION: 826.7 (MSL) EOB: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 50.0 ft. 1 OF 2 SAMPLING METHOD: SPT / ST START: 8/12/21 END: 8/12/21 **ENERGY RATIO (%):** 89 LAT / LONG: 41.431063, -81.598479 **MATERIAL DESCRIPTION** ELEV. HP **GRADATION (%) ATTERBERG** SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 826.7 4 L 100 MEDIUM DENSE TO VERY DENSE, BROWN AND BLACK. SANDY SILT. LITTLE GRAVEL. LITTLE CLAY. CONTAINS 98800 S BRICK FRAGMENTS AND TRACE IRON STAINING, DAMP 2 (FILL) 3 58 SS-1 72 A-4a (V) 17 12 5 39 SS-2 20 20 23 5 15 20 19 21 16 A-4a (1) 14 6 819.2 HARD, GRAYISH BROWN, CLAY, SOME SILT, TRACE 8 GRAVEL, TRACE SAND, MOIST 2 27 4.25 5 65 43 23 20 25 A-7-6 (13) 79 ST-3 1 1 × 9 817.2 à Valen VERY DENSE, BROWN, STONE FRAGMENTS WITH SAND, 68 SS-4 A-1-b (V) 44 TOP W LITTLE SILT. TRACE CLAY, STONE FRAGMENTS ARE SANDSTONE. CONTAINS IRON STAINING. DAMP NOD 1 814.9 12 LOOSE, BROWN, COARSE AND FINE SAND, SOME SILT, TRACE CLAY, TRACE GRAVEL, CONTAINS TRACE IRON 2 > 2008 13 STAINING, WET 2 6 78 SS-5 17 A-3a (V) 14 812.2 CONP < **W** 811.7 MEDIUM DENSE TO VERY DENSE, BROWN AND GRAY, 15 933000 S **GRAVEL AND STONE FRAGMENTS WITH SAND. TRACE TO** action 1 27 44 SS-6 A-1-b (V) 9 LITTLE SILT, TRACE CLAY, MOIST TO WET 16 @17.5' TO 50.0'; ENCOUNTERED COBBLES DURING 18 NP 15 53 100 SS-7 28 44 14 10 4 NP NP 12 A-1-b (0) DRILLING 19 20 14 50 15 33 SS-8 14 A-1-b (V) 21 22 N N 10 49 13 44 SS-9 A-1-b (V) 20 S Wash 24 400 A The 1 25 J 500 1 3 56 72 SS-10 A-1-b (V) 16 26 20 > \$277X/S 27 1 1 1 1 28 62 39 19 SS-11 13 A-1-b (V) 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 1000 < 10 23 29 (1800m) S × 47/20/



EXPLORATION ID DRILL RIG: PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES CME 75T STATION / OFFSET: 380+40, 65' LT. B-014-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 827.2 (MSL) EOB: 50.0 ft. 1 OF 2 SPT START: 8/13/21 END: 8/13/21 SAMPLING METHOD: **ENERGY RATIO (%):** LAT / LONG: 41.431281, -81.598355 ELEV. HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD GR CS FS SI CL WC FILL AND NOTES (%)ID (tsf) LL PL Ы 827.2 MEDIUM DENSE TO VERY DENSE, GRAY AND BROWN. 12 46 27 8 NP NP 56 SS-1 41 21 3 NP A-1-b (0) **GRAVEL WITH SAND**, TRACE SILT, TRACE CLAY, WET TO Washing & 19 MOIST 2 36 NP 10 44 SS-2 52 17 19 9 3 NP NP 12 A-1-b (0) 3 62 67 SS-3 A-1-b (V) 19 13 SS-4A 12 A-1-b (V) 821.7 16 83 5 SS-4B 2.75 29 A-7-6 (V) VERY STIFF, BROWN, CLAY, SOME SILT, TRACE SAND, 6 TRACE GRAVEL. MOIST 3.00 3 3 29 21 24 23 A-7-6 (15) SS-5A 4 61 45 820.2 10 78 4 SS-5B 8 A-1-b (V) LOOSE, BROWN, GRAVEL WITH SAND, TRACE SILT. 819.2 TRACE CLAY, DAMP 8 VERY DENSE. BROWN. GRAVEL AND STONE 1 de 10 9 FRAGMENTS, TRACE SAND, TRACE SILT, TRACE CLAY, 200 56 56 SS-6 18 A-1-a (V) POSSIBLE ENCOUNTER WITH COBBLE. DAMP 816.7 TOTAL STORY STIFF. BROWN AND ORANGISH BROWN. SANDY SILT. LITTLE CLAY, LITTLE GRAVEL, CONTAINS IRON STAINING, N 800 1 6 18 56 SS-7 1.25 11 23 28 23 15 24 14 10 17 A-4a (1) MOIST 2 > 2008 13 1 1 5 813.6 I stored " MEDIUM DENSE. BROWN AND GRAY. COARSE AND FINE 27 33 SS-8 A-3a (V) 10 SUND S SAND, SOME SILT, LITTLE CLAY, LITTLE GRAVEL, MOIST 11 15 933000 S 811.7 ZZ , 47/2// DENSE TO VERY DENSE, GRAY, GRAVEL WITH SAND, 16 TRACE SILT, TRACE CLAY, MOIST TO WET 52 SS-9 15 78 A-1-b (V) 11 @15.5' TO 50.0'; DIFFICULT DRILLING/ENCOUNTERED COBBLES DURING DRILLING 18 OH DOT.GDT 16 19 62 22 SS-10 A-1-b (V) 21 20 21 49 SS-11 60 15 8 NΡ NP NP A-1-b (0) 50 13 4 22 10 S Valen 24 56 18 56 SS-12 A-1-b (V) 11 25 JOD J 26 Samon & 2 > 2008 27 799.7 VERY DENSE. BROWN BECOMING GRAY. GRAVEL AND 28 STONE FRAGMENTS. SOME SAND. TRACE SILT. TRACE CLAY, MOIST 29 WBATTAL S



EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 381+46, 28' RT. B-015-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14 PAGE** PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 829.0 (MSL) EOB: 45.0 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/11/21 END: 8/11/21 **ENERGY RATIO (%):** 89 LAT / LONG: 41.430932, -81.598114 **MATERIAL DESCRIPTION** ELEV. HP **GRADATION (%) ATTERBERG** SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL ы WC FILL AND NOTES ID (tsf) 829.0 4 L 100 VERY LOOSE TO MEDIUM DENSE, BROWN, COARSE AND FINE SAND, LITTLE GRAVEL, LITTLE SILT, TRACE CLAY, 98800 S MOIST 2 3 13 100 SS-1 5 8 A-3a (V) 5 22 NP NP 89 SS-2 18 34 16 10 NP 11 A-3a (0) 6 822.0 VERY STIFF, BROWN, CLAY, "AND" SILT, LITTLE SAND, TRACE GRAVEL, MOIST 8 6 100 SS-3 A-7-6 (V) 3.50 9 S Vals 10 TOTAL AND 2 100 SS-4 3.25 7 10 40 42 50 23 27 A-7-6 (17) 1 30 J 8 7 7 817.0 12 **W** 816.5 MEDIUM DENSE TO VERY DENSE, BROWN BECOMING [0 Qq 2 > 2008 GRAY, GRAVEL AND STONE FRAGMENTS. SOME SAND. 13 22 2 NP NP 19 58 44 SS-5 57 11 8 NP A-1-a (0) TRACE SILT, TRACE CLAY, WET TO DAMP 14 CONP < 15 933000 S 13 @15.0' - 45.0'; ENCOUNTERED COBBLES DURING DRILLING autim 1 25 28 SS-6 A-1-a (V) 10 16 17 18 22 68 56 SS-7 A-1-a (V) 16 24 19 20 15 82 100 SS-8 8 A-1-a (V) 21 22 23 24 400 A 25 N 800 1 SS-9 2 NP NP NP 20 67 100 64 19 9 6 8 A-1-a (0) 26 20 > \$277X/S 27 28 29 933000 S × 47/20/

Р	ID: 104132	SFN:		PROJECT:	CUY-1	4-6.93	STA	TION /	OFFSE	T::	381+4	6, 28' RT.	_ s	TART	: 8/1	1/21	_ EN	ND: _	8/1	1/21	_ P(G 2 OF	2 B-01	5-0-21
			MATERIAL DESCRIP AND NOTES	PTION		ELEV.	DEPTHS	S	SPT/ RQD	N ₆₀		SAMPLE ID	HP (tof)		GRAD			_		ERBE PL	RG PI	wc	ODOT CLASS (GI)	BACK FILL
(GRAY, Grave	L AND	VERY DENSE, BROV D STONE FRAGMENT CLAY, WET TO DAM	'S , SOME SAND,		799.0	-	31	17 23 21		100	SS-10	(tsf)	GR -	cs -	FS -	SI -	CL -	-	-	- -	9	A 4 - 0.0	
		. 0 (0)	CEN, WEI TO DAW	(commod)			-	- 32 — - 33 — - 34 —																
							-	- 35 — - 36 — - 37 — - 38 — - 39 —	21 <u>- 50/2"</u> -	-	25	SS-11	-	-	-	-	-	-	•	-	-	2	A-1-a (V)	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
							-	- 40 — - 41 — - 42 — - 43 —	23 24 27	76	22	SS-12	-	-	-	-	-	-	-	-	-	11	A-1-a (V)	1 V 1
						784.0	EOB	- 44 -45	22 26 31	85	17	SS-13	-	-	-	-	-	-	-	-	-	11	A-1-a (V)	

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 382+01, 13' LT. B-016-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE ELEVATION: 831.0 (MSL) EOB: PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 35.0 ft. 1 OF 2 SAMPLING METHOD: SPT / ST START: 8/11/21 END: 8/11/21 **ENERGY RATIO (%):** 89 LAT / LONG: 41.430972, -81.597870 **MATERIAL DESCRIPTION** ELEV. HP **GRADATION (%) ATTERBERG** SPT/ REC SAMPLE **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD GR CS FS SI CL LL PL Ы WC FILL AND NOTES (%)ID (tsf) 831.0 DENSE, BLACK AND BROWN, GRAVEL WITH SAND, LITTLE 37 16 NP NP 14 83 SS-1 42 20 16 6 NP A-1-b (0) SILT, TRACE CLAY, SLIGHTLY ORGANIC, MOIST (1800m) S 829.5 ZZ , 47/2// MEDIUM STIFF TO VERY STIFF, BROWN AND GRAY, CLAY, 2 3 10 7 22 100 SS-2 2.75 5 8 37 43 49 27 24 A-7-6 (17) LITTLE TO "AND" SILT, TRACE TO LITTLE SAND, TRACE GRAVEL, CONTAINS IRON STAINING, MOIST TO DAMP 3 2 7 56 SS-3 2.00 28 A-7-6 (V) 5 2 4 100 SS-4 0.75 3 2 2 72 52 24 A-7-6 (18) 21 28 29 6 4 100 SS-5 1.00 A-7-6 (V) @7.8' TO 8.3'; Qu = 1409 PSF @ 13.5% 8 3.50 0 17 81 54 24 30 32 A-7-6 (19) 77 ST-6 1 9 2 100 SS-7 2.00 A-7-6 (V) THE WAY NODI: 12 2 > 2008 13 5 16 100 SS-8 2.75 A-7-6 (V) 816.5 CONP < MEDIUM DENSE, BROWN AND GRAY, GRAVEL AND 15 933000 S STONE FRAGMENTS WITH SAND AND SILT. TRACE CLAY. 2 . ATTEL 21 89 **SS-9** 51 18 10 13 8 26 18 8 11 A-2-4 (0) 7 CONTAINS IRON STAINING, DAMP 16 814.0 17 DENSE TO VERY DENSE, GRAY, GRAVEL AND STONE FRAGMENTS. "AND" SAND. TRACE SILT. TRACE CLAY. 18 000 13 45 72 SS-10 A-1-a (V) 11 DAMP TO MOIST 17 19 20 59 NP NP 19 83 SS-11 50 31 11 1 NP 9 A-1-a (0) 21 22 23 24 TOTAL STORY 25 N 000 1 3 SS-12 39 122 100 A-1-a (V) 26 43 2 > 2008 27 28 29 WBATTAL S

	PID: <u>104132</u>	SFN:	PROJECT:	CUY-1	4-6.93		STATION /	OFFSE	T:	382+0	1, 13' LT.	_ s	START	: 8/	11/21	_ EN	ND: _	8/1	1/21	_ P	G 2 OI	- 2 B-01	6-0-21
3/6		MATERIAL DESCRIP	TION		ELEV.	חר	PTHS	SPT/	NI.	REC	SAMPLE	HP		GRAD	ATIC	N (%)	ATT	ERBE	≣RG		ODOT	BACK
ە ق		AND NOTES			801.0	DE	PINS	RQD	N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	Ц	PL	PI	WC	CLASS (GI)	FILL
\CUY-14		RY DENSE, GRAY, GRAVEL 'AND" SAND, TRACE SILT, 'ST. (continued)		000			- - 31 -	17 29 29	86	89	SS-13	-	-	-	-	-	1	-	-	1	7	A-1-a (V)	
1 ARCHIVE	DAINIP TO MOIS	or (continuea)					- 32 - - 33 -																1 N N N N N N N N N
EAR\Z0Z					796.0	—FOB	- 34 -	20 27 33	89	44	SS-14	-	-	-	-	-	-	-	-	-	7	A-1-a (V)	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE. RAN A SLURRY MIX AS CIRCULATING FLUID.

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / ASHBAUGH DRILL RIG: CME 55T STATION / OFFSET: 383+04, 46' RT. B-017-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / ASHBAUGH HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: ELEVATION: 845.8 (MSL) EOB: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 41.5 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/24/21 END: 8/24/21 **ENERGY RATIO (%):** 68.4 LAT / LONG: 41.430711, -81.597615 MATERIAL DESCRIPTION ELEV. SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ REC **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC FILL AND NOTES ID (tsf) 845.8 7.0" ASPHALT AND 7.5" CONCRETE (DRILLERS 844.6 DESCRIPTION) 8 L 400 STIFF TO VERY STIFF, BROWN AND GRAY, CLAY, SOME WSOM " 2 SILT. TRACE TO LITTLE GRAVEL. TRACE TO LITTLE SAND. ZZ PILEL CONTAINS TRACE IRON STAINING, MOIST 3 8 SS-1 A-7-6 (V) 3 44 2.00 26 5 9 2.25 56 SS-2 27 A-7-6 (V) 6 @7.5'; SS-3 CONTAINS A 2.0" STONE FRAGMENT A-7-6 (15) 8 3 50 SS-3 3.00 16 2 26 52 45 20 25 4 26 \$ > \rightarrow \frac{1}{2} 9 10 2 6 SS-4 1.75 A-7-6 (V) 56 25 S July THE WAY 833.8 12 MEDIUM DENSE, BROWN, GRAVEL AND STONE JODD J. FRAGMENTS WITH SAND. LITTLE SILT. TRACE CLAY. 13 NP 23 17 NP 5 15 28 SS-5 43 11 6 NP A-1-b (0) DAMP 3 > MANA 14 831.3 VERY DENSE, GRAY, STONE FRAGMENTS, TRACE SAND, 15 50/3" - A 67 A SS-6 A-1-a (V) TRACE SILT, TRACE CLAY, POSSIBLE COBBLE(S), DAMP TO 000 DRY 16 88M2 5 17 507 2 A-1-a (V) 50/3" - 67 SS-7 GDT ALXD< 19 826.3 VERY STIFF TO HARD. GRAY AND BROWN. CLAY. SOME 20 SILT, TRACE SAND, TRACE GRAVEL, DAMP TO MOIST 17 67 SS-8 2.50 22 A-7-6 (V) 21 22 Land A-7-6 (14) 24 3 34 22 9 72 SS-9 4.50 4 4 55 44 22 22 24 25 10 400 A 10 26 83 SS-10 4.50 20 A-7-6 (V) 26 Valor 13 1000 J 27 817.8 SS-11A 2.50 23 A-7-6 (V) 23 > 2000/8 28 29 11 56 SS-11B 12 A-1-b (V) 14 29 < 47XP

	PID: <u>104132</u>	SFN:	PROJECT: CUY-	14-6.93		STATION /	OFFSE	T:	383+0	4, 46' RT.	_ S	TART:	8/2	4/21	EN	ND: _	8/24	1/21	_ P	G 2 OF	2 B-01	7-0-21
		MATERIAL DESCRIPT	TION	ELEV.	DF	PTHS	SPT/	N ₆₀	l	SAMPLE	HP		SRAD.			-	ATT	ERBE			ODOT	BACK
L		AND NOTES		815.8			RQD	• •60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL
	GRAVEL AND	SE TO VERY DENSE, BROW STONE FRAGMENTS WITH LAY, SS-11B CONTAINS TR	SAND, LITTLE			- - 31 -	13 14 14	32	44	SS-12	-	-	-	-	-	-	-	-	-	12	A-1-D(V)	DZ 901/2011
l		MP TO MOIST (continued)	ACE INON]		_ 32 -																1 > 1 L
			5°-{\cdot\cdot\cdot\cdot\cdot\cdot\cdot\cdot			- 33 - - - 34 -	17 21 27	55	78	SS-13	-	43	22	18	14	3	NP	NP	NP	8	A-1-b (0)	1 L' 1 D
						_ 35 ¬																LVespl
						- 36 -	_15 22 29	58	67	SS-14	-	-	-	-	-	-	-	-	-	11	A-1-b (V)	77777
						_ 37 _																422
	@37.5'; SS-15 (CONTAINS NO RECOVERY				- 38 - - 39	20 26 31	65	0	SS-15	-	-	-	-	-	-	-	-	-	-	A-1-b (V)	1 > N > N > N > N > N > N > N > N > N >
						- 40																
				804.3	—EOB	- 41 -	22 25 35	68	44	SS-16	-	-	-	-	-	-	-	-	-	10	A-1-b (V)	7 1 E
ļ					—EOB																	

	ORILLING FIRM / OPERA	TOR: N	EAS / ASHBAL	UGH_		L RIG:		CME 5			STAT	ION	OFF	SET:	3	84+07	7, 19'	LT.	EXPLOR	
	SAMPLING FIRM / LOGG			JGH_		MER:		IE AUTOI			ALIG					SR-14				3-0-21
	ORILLING METHOD:	3.	25" HSA			BRATI			2/5/19				N: 8						2.8 ft	PAGE
START: <u>8/24/21</u> END: <u>8/24/21</u> S	SAMPLING METHOD:		SPT		ENEF	RGY R			68.4	_	LAT /							.59717	74	1 OF 1
MATERIAL DESCRIPTIO	N	ELEV.	DEPTHS		SPT/	N ₆₀		SAMPLE					N (%)	_		ERBE			ODOT	BACK
AND NOTES	N A A	843.8	DEI IIIO		RQD	' 1 60	(%)	ID	(tsf)	GR	cs	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	FILL
12.0" ASPHALT AND 6.0" BASE (DRILLERS I																				
		842.3		1 —																7 LV 7 L
STIFF TO VERY STIFF, BROWNISH GRAY AN	ND BROWN, RAVEL, SS-2		<u> </u>	2 —																1>11>
CLAY, SOME SILT, TRACE SAND, TRACE GR CONTAINS A 1.5" CONCRETE FRAGMENT. M	MOIST			3	3															1 L 1 L
(FILL)				Ĭ	3 4	8	56	SS-1	4.00	5	3	5	35	52	42	20	22	24	A-7-6 (13)	1>V 1>
				4																1>11>
			<u> </u>	5	1															1 LV 1 L
				6	3	8	22	SS-2	2.75	-	-	-	-	-	-	-	-	22	A-7-6 (V)	1>V 1>
				- 4	4															1>11>
				7 —																JLV JL
			<u> </u>	8	4 4	11	61	SS-3	2.25	4	4	6	29	57	44	20	24	25	A-7-6 (14)	1>11>
				9	⁴ 6	' '	01	33-3	2.23	_ +	4		29	31	44	20	24	23	A-1-0 (14)	1>1 1>
				9 -																1 LV 1 L
				10	5															1>11>
				11	5 5	11	78	SS-4	2.00	-	-	-	-	-	-	-	-	24	A-7-6 (V)	1 LV 1 L
		831.8		12 —																12V 17
STIFF TO HARD, BROWN, SILT , SOME SAND	D, LITTLE CLAY, ‡‡‡‡‡		l F		6															1>11>
TRACE GRAVEL, CONTAINS IRON STAINING MOIST	i, DAMP TO			13	6 5	13	67	SS-5	4.50	_	-	-	-	-	-	_	-	13	A-4b (V)	1 LV 1 L
(FILL)	+ + + +			14	6															1>11>
()	++++			45																1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 × 1 ×
	++++			¹⁵ T	2	_		00.0	0.00			40		40	- 00	40		00	A 41: (0)	1 LV 1 L
	+ + + +		<u> </u>	16	2	6	56	SS-6	2.00	3	3	18	57	19	26	18	8	22	A-4b (8)	1>11>
	++++			17 -																1 LV 1 L
	++++		-	-	4															1>V 1>
;	++++			18	3	8	44	SS-7	1.75	-	-	-	-	-	-	-	-	20	A-4b (V)	1>11>
	+ + + + + + + +	824.3	<u> </u>	19	4															1 LV 1 L
LOOSE, BROWN, GRAVEL WITH SAND , LITT	LE SILT,			20	•															1>/1>
TRACE CLAY, DAMP	$\mathbb{P}^{\mathbb{Q}}$		l -		3	9	56	SS-8	_	l _	_	_	_	_	_	_	_	8	A-1-b (V)	1>1 1>
(FILL)		004.0		21	5															JLV JL
VERY DENSE, RED, STONE FRAGMENTS (B I	RICK) TRACE	821.8	<u> </u>	22 —																1>11>
SAND, TRACE SILT, TRACE CLAY, CONTAINS		821.0	L_EOB		حر "50/4	لمت	<u>50</u>	SS-9	متہ	Ŀ	لتا	ــــــ	لمتا		ــــا	لت	لت	_5_	A-1-a (V)	5.V 5.
FRAGMENT, DAMP																				
(FILL)																				
NOTES: GROUNDWATER NOT ENCOUNTE	RED DURING DRILLING.	HOLE D	ID NOT CAVE	. BORI	NG OF	FSET	1 <u>0.0'</u> N	NE DUE T	O UTIL	<u>ITIE</u> :	S									

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.5 BAG ASPHALT PATCH; SHOVELED SOIL CUTTINGS

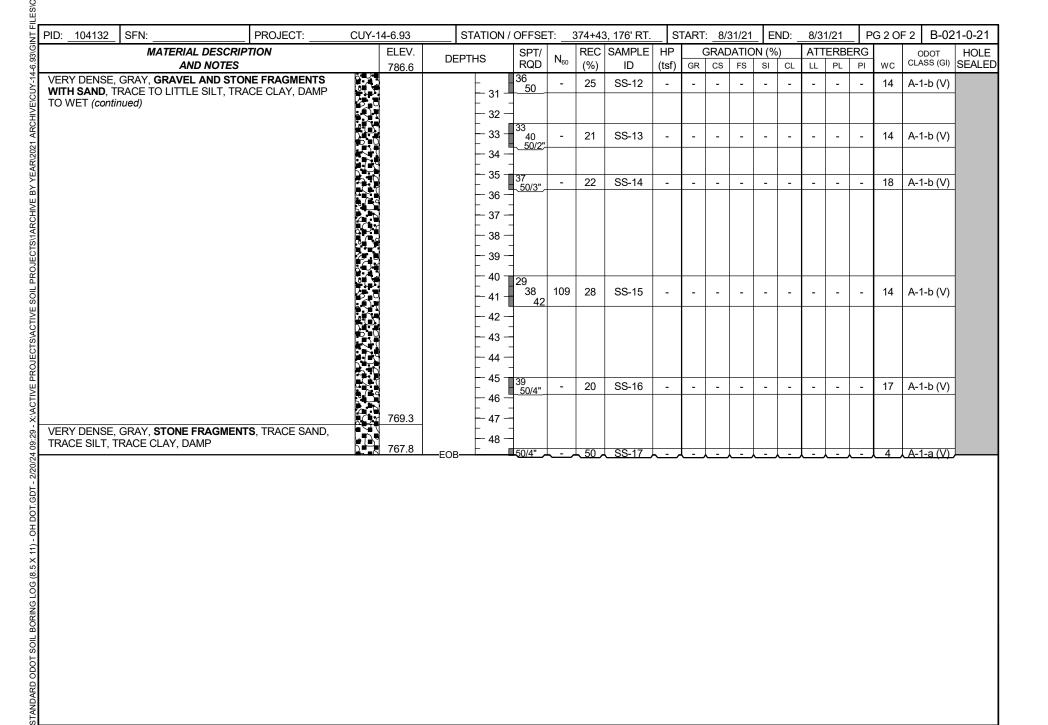
PID: 104132 SFN:	DRILLING FIRM / OPERA SAMPLING FIRM / LOGG DRILLING METHOD: SAMPLING METHOD:	ER:	NEAS / JL NEAS / JL 25" HSA SPT	HAM CALI					_	STAT ALIG ELEV	NMEI /ATIC)N: _8	343.7	(MSI		OB:		.5 ft.	ATION ID 9-0-21 PAGE 1 OF 1
MATERIAL DESCRIPTI AND NOTES		ELEV. 843.7	DEPTHS	SPT/ RQD			SAMPLE ID	_	(GRAD	_	N (%)	_	_	ERBE PL	_	wc	ODOT CLASS (GI)	BACK FILL
3.5" TOPSOIL (DRILLERS DESCRIPTION) VERY STIFF TO HARD, BROWN AND GRAY SILT, TRACE GRAVEL, TRACE SAND, MOIS	, CLAY, LITTLE	843.4	- 1 - - 1 -	3															\\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
	C, CLAY, LITTLE		_ 2 _ _ 3 _	4 6 3	11	61	SS-1	3.75	3	1	1	15	80	52	25	27		A-7-6 (17)	1>\\ 1\\ 1\\ 1\\ 1\\ 1\\ 1\\ 1\\ 1\\ 1\\
			- 4 - - - 5 -	3 5	10	67 72	SS-2 SS-3	3.75	1	0	0	20	79	56	26	30	30	A-7-6 (19) A-7-6 (V)	1
		836.2	- 6 - - 7 -	4 4 5 4	10	89	SS-4	4.25		-	-	-	-	-	-	-	28	A-7-6 (V)	1>1 1> 1 LV 1 L

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING. HOLE DID NOT CAVE.

ABANDONMENT METHODS, MATERIALS, QUANTITIES: SHOVELED SOIL CUTTINGS

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 75T STATION / OFFSET: 373+66, 148' RT. B-020-0-21 TYPE: **RETAINING WALL** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 5/1/19 ELEVATION: 816.2 (MSL) EOB: 45.0 ft. 1 OF 2 SAMPLING METHOD: SPT **ENERGY RATIO (%):** LAT / LONG: 41.430969, -81.600935 START: 8/6/21 END: 8/6/21 89 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ HOLE ODOT **DEPTHS** N_{60} CLASS (GI) RQD SEALED (%) GR CS FS SI CL LL PL ы WC AND NOTES ID (tsf) 816.2 MEDIUM DENSE, BROWN AND GRAY, GRAVEL WITH SAND AND SILT, TRACE CLAY, DAMP 2 3 SS-1 33 25 16 18 8 NP NP NP 28 50 7 A-2-4 (0) 10 811.7 SOIL PROJECTS/1ARCHIVE VERY LOOSE TO LOOSE, BROWN, COARSE AND FINE 5 SAND, LITTLE SILT, LITTLE GRAVEL, TRACE CLAY, 6 SS-2 2 100 A-3a (V) 16 CONTAINS INTERBEDDED 1.0" SILTY CLAY SEAMS, WET 6 8 2 89 SS-3 25 35 19 10 NP NP NP 18 11 A-3a (0) 9 806.7 w 806.2 MEDIUM DENSE, BROWN, STONE FRAGMENTS, SOME 10 SAND. LITTLE SILT. TRACE CLAY, MOIST TO WET 21 28 SS-4 6 12 A-1-a (V) @10.0' - 45.0'; ENCOUNTERED COBBLES DURING DRILLING 12 13 NP NP 22 12 12 3 NP 56 SS-5 59 14 A-1-a (0) 14 801.7 MEDIUM DENSE TO VERY DENSE, GRAY, GRAVEL WITH 15 13 SAND. TRACE SILT. TRACE CLAY, WET TO MOIST STANDARD ODOT SOIL BORING LOG (8.5 X 11) - OH DOT.GDT - 2/20/24 09:29 28 100 SS-6 A-1-b (V) 12 16 17 18 18 58 100 SS-7 13 A-1-b (V) 19 20 12 55 18 33 SS-8 11 A-1-b (V) 21 22 14 23 59 18 33 SS-9 A-1-b (V) 24 25 16 SS-10 36 15 10 3 NP NP NP 18 59 50 36 11 A-1-b (0) 26 27 8 28 64 28 21 SS-11 12 A-1-b (V) 22 29

EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / J. HODGES DRILL RIG: CME 55X STATION / OFFSET: 374+43, 176' RT. B-021-0-21 TYPE: **CULVERT** SAMPLING FIRM / LOGGER: NEAS / J. HODGES HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 816.6 (MSL) EOB: 48.8 ft. 1 OF 2 SAMPLING METHOD: SPT / ST START: 8/31/21 END: 8/31/21 **ENERGY RATIO (%):** 81.9 LAT / LONG: 41.430865, -81.600658 **MATERIAL DESCRIPTION** ELEV. SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ REC HOLE ODOT **DEPTHS** N_{60} CLASS (GI) RQD (%) GR CS FS SI CL LL PL Ы WC SEALED AND NOTES ID (tsf) 816.6 VERY STIFF, BROWN, SANDY SILT, LITTLE GRAVEL. LITTLE CLAY, CONTAINS IRON STAINING, DAMP TO MOIST 2 3 SS-1 29 15 22 11 100 2.50 19 18 19 15 7 4 11 A-4a (2) 5 2 100 SS-2 3.00 19 A-4a (V) 6 809.6 7 SOFT TO STIFF, BROWN, SILT AND CLAY, SOME SAND, SOME GRAVEL, DAMP 8 808.2 @7.6' TO 8.1'; Qu = 582 PSF @ 3.5% 22 1.25 23 18 16 21 28 17 100 ST-3 11 15 A-6a (2) @8.2'; CONTAINS A 3.0" BLACK ORGANIC SEAM 9 807.2 CONTAINING WOOD FRAGMENTS 44 100 SS-4 0.25 BROWNISH GRAY WITH TRACE BLACK STREAKS, 15 A-7-6 (V) COARSE AND FINE SAND, LITTLE SILT, TRACE GRAVEL, 17 TRACE CLAY, SLIGHTLY ORGANIC, WET 804.8 12 SOFT, DARK GRAY, CLAY, SOME SILT, LITTLE SAND, TRACE GRAVEL, CONTAINS IRON STAINING, MOIST 13 NP 15 NP NP 18 67 100 SS-5 39 24 16 6 A-1-b (0) VERY DENSE, GRAY, GRAVEL AND STONE FRAGMENTS 31 WITH SAND, TRACE TO LITTLE SILT, TRACE CLAY, DAMP 15 @11.8' TO 48.8'; ENCOUNTERED COBBLES AND/OR **BOULDERS DURING DRILLING** 64 39 SS-6 A-1-b (V) 15 16 17 18 36 96 44 SS-7 A-1-b (V) 19 20 94 NP NP 28 56 SS-8 41 28 16 12 3 NP 10 A-1-b (0) 21 41 22 20 SS-9 A-1-b (V) 50/3 24 25 40 SS-10 15 A-1-b (V) 50/4" 26 27 28 29 SS-11 13 A-1-b (V) 50/2 29



EXPLORATION ID PROJECT: CUY-14-6.93 DRILLING FIRM / OPERATOR: NEAS / ASHBAUGH DRILL RIG: CME 55T STATION / OFFSET: 374+00, 292' RT. B-022-0-21 TYPE: **CULVERT** SAMPLING FIRM / LOGGER: NEAS / ASHBAUGH HAMMER: CME AUTOMATIC ALIGNMENT: **SR-14** PAGE PID: 104132 SFN: DRILLING METHOD: 3.25" HSA CALIBRATION DATE: 12/5/19 ELEVATION: 849.3 (MSL) EOB: 51.5 ft. 1 OF 2 SAMPLING METHOD: SPT START: 8/26/21 END: 8/26/21 **ENERGY RATIO (%):** 68.4 LAT / LONG: 41.430563, -81.600868 ELEV. SAMPLE HP **GRADATION (%) ATTERBERG** MATERIAL DESCRIPTION SPT/ REC **BACK** ODOT **DEPTHS** N_{60} CLASS (GI) RQD GR CS FS SI CL LL PL Ы WC FILL AND NOTES (%)ID (tsf) 849.3 4.0" TOPSOIL (DRILLERS DESCRIPTION) 849.0/ 22 NP NP NP 44 SS-1 39 26 21 11 3 A-1-b (0) MEDIUM DENSE TO VERY DENSE. BROWN. GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE 2 CLAY, SS-1 CONTAINS ROOT HAIRS, DAMP TO MOIST 24 10 44 SS-2 10 A-1-b (V) 3 29 28 SS-3 62 11 11 13 3 NP NP NP A-1-b (0) 14 10 5 46 50 SS-4 A-1-b (V) 19 5 6 43 56 SS-5 A-1-b (V) 10 8 47 67 SS-6 29 38 15 14 NP NP NP A-1-b (0) 4 24 9 10 18 55 SS-7 A-1-b (V) 56 12 13 52 21 61 SS-8 A-1-b (V) 834.8 VERY DENSE, LIGHT BROWN, COARSE AND FINE SAND, 15 イント 12 TRACE GRAVEL, TRACE SILT, TRACE CLAY, DAMP 57 61 SS-9 A-3a (V) 21 16 29 832.3 17 DENSE TO VERY DENSE, BROWN, GRAVEL AND STONE FRAGMENTS. SOME SAND. TRACE SILT. TRACE CLAY. 18 000 NP 15 49 50 SS-10 52 18 16 10 4 NP NP 5 A-1-a (0) 11) - OH DOT.GDT DAMP 19 20 84 33 SS-11 A-1-a (V) 21 40 22 20 79 22 SS-12 A-1-a (V) 40 Pood 24 824.8 VERY DENSE, BROWN, COARSE AND FINE SAND, TRACE 25 12 SILT. TRACE CLAY, TRACE GRAVEL, DAMP NP NP 58 50 SS-13 29 56 10 4 NP A-3a (0) 1 6 26 30 1>1 27 5 28 52 56 20 SS-14 A-3a (V) 26 29 819.8 1>1

PID): _	1041	132_	SFN: _			PROJECT:	CUY-1	14-6.93		STATION	N / OFFSE	ET:;	374+00), 292' RT.	_ S	TART	: 8/2	6/21	EN	ND: _	8/26	6/21	_ P	G 2 OF	2 B-02	22-0-21
				M	ATERIAL D		ON		ELEV.	D	EPTHS	SPT/	N ₆₀		SAMPLE		-	GRAD			,		ERBE			ODOT	BAC
L.,		<u> </u>				NOTES		E & Je (819.3			RQD	60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	
							NE FRAGMENTS AMP (continued)		817.3		31	50/2"	-	63	SS-15	-	-	-	-	-	-	-	-	-	5	A-1-b (V)	V
							E AND FINE CE CLAY, DAMP				- 32 - 33 - 34	12	49	67	SS-16	-	-	-	-	-	-	-	-	-	6	A-3a (V)	1
											- 35 - 36 - 37	21	58	83	SS-17	-	-	-	-	-	-	-	-	-	7	A-3a (V)	1
					I, GRAVEL LT, TRACE		NE FRAGMENTS DIST		811.0	W 81	11.1 — 38 — 39	-															<pre></pre> <pre><</pre>
											- 40 - 41 - 40	16 24 31	63	50	SS-18	-	50	21	13	12	4	NP	NP	NP	9	A-1-b (0)	1 > \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \
					I, GRAVEL , DIST TO WI		ND, TRACE			_	42 43 44	-															V
											45 46 47	14 23 30	60	67	SS-19	-	52	25	12	8	3	NP	NP	NP	9	A-1-a (0)	1 × 1 × 1 × 1 × 1
											- 48 - 49 - 50	20															7
								000	797.8	EO	_ 51	22	65	72	SS-20	-	-	-	-	-	-	-	-	-	13	A-1-a (V)	7 > 7 7

APPENDIX C

GEOTECHNICAL BULLETIN 1 (GB1) ANALYSIS SPREADSHEETS



OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

CUY-14-6.93 104132 SR-14 (366+40 to 368+85) (Replacement of existing Bridge CUY-14-0693)

NEAS Inc.

Prepared By: Date prepared:

Brendan P Andrews, PE

Monday, February 13, 2023

Brendan P. Andrews 1329 East Kemper Road

Suite 4104B

Cincinnati, OH 45246 513.337.9823 Ext. 701

brendan.andrews@neasinc.com

NO. OF BORINGS:

1

1/18/2019



#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring	Proposed Subgrade EL	Cut Fill
1	B-001-0-21	SR-14	368+69	29	Right	CME 55T	68	831.3	832.2	0.9 F





1/18/2019

#	Boring	Sample	San De	iple pth	Subg De _l		Stan Penet	dard ration	НР		Pl	nysica	al Chara	cteristics		Moi	sture	Ohio	DOT	Sulfate Content	Proble	m	Excavate an (Item	•	Recommendation (Enter depth in
"			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	1.0	4.0	1.9	4.9	10									10	6	A-1-b	0						
	001-0	SS-2	5.0	6.5	5.9	7.4	15		4.25	23	16	7	46	21	67	15	11	A-4a							
	21																								
								10																	

OHIO DEPARTMENT OF **TRANSPORTATION**



PID: 104132

County-Route-Section: CUY-14-6.93

No. of Borings: 1

Geotechnical Consultant: NEAS Inc.

Prepared By: Brendan P Andrews, PE

Date prepared: 2/13/2023

C	Chemical Stabilization Option	IS
320	Rubblize & Roll	No
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	14"

Excavate and Repl	ace
Stabilization Option	ons
Global Geotextile	
Average(N60L):	12"
Average(HP):	0''
Global Geogrid	
Average(N60L):	0''
Average(HP):	0''

Design CBR	13
---------------	----

% Samples within 6 feet of subgrade										
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%							
N ₆₀ < 12	50%	0.5 < HP ≤ 1	0%							
12 ≤ N ₆₀ < 15	0%	1 < HP ≤ 2	0%							
N ₆₀ ≥ 20	0%	HP > 2	50%							
M+	0%									
Rock	0%									
Unsuitable	0%									

Excavate and Replace at Surface								
Average	0"							
Maximum	0"							
Minimum	0"							

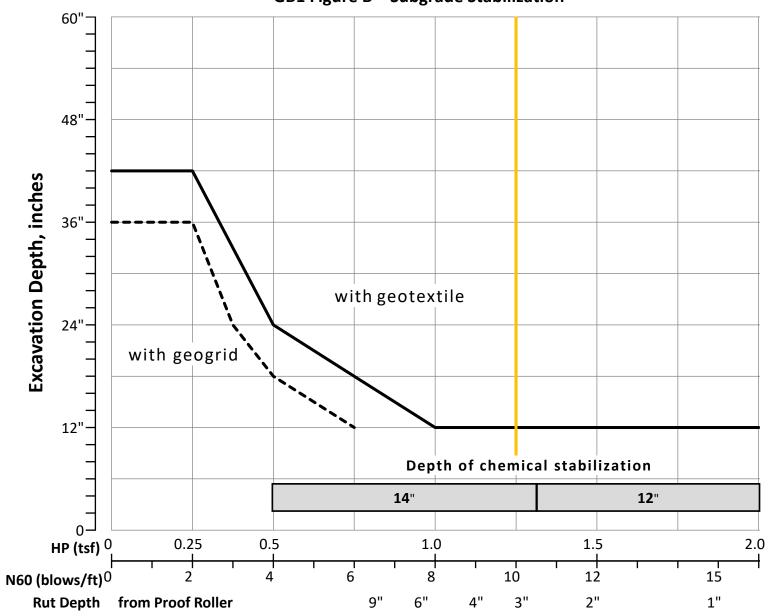
% Proposed Subgrade Surface								
Unstable & Unsuitable	0%							
Unstable	0%							
Unsuitable	0%							

	N ₆₀	N _{60L}	HP	Ш	PL	PI	Silt	Clay	P 200	M _c	M _{OPT}	GI
Average	13	10	4.25	23	16	7	46	21	67	13	9	0
Maximum	15	10	4.25	23	16	7	46	21	67	15	11	0
Minimum	10	10	4.25	23	16	7	46	21	67	10	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	1	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	2
Percent	0%	0%	50%	0%	0%	0%	0%	0%	0%	50%	0%	0%	0%	0%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%		100% 0%									100%							
Surface Class Count	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
Surface Class Percent	0%	0%	100%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization



OVERRIDE TABLE

Calculated Average	New Values	Check to Override
4.25	0.50	HP
10.00	6.00	N60L



OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

CUY-14-6.93 104132

SR-14 (382+00 to 385+40) / Old Broadway Ave (Replacement of existing Bridge CUY-14-0693)

NEAS Inc.

Prepared By: Date prepared:

Brendan P Andrews, PE

Monday, February 13, 2023

Brendan P. Andrews 1329 East Kemper Road

Suite 4104B

Cincinnati, OH 45246 513.337.9823 Ext. 701

brendan.andrews@neasinc.com

NO. OF BORINGS:

3





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER		Proposed Subgrade EL	Cut Fill
1	B-017-0-21	SR-14	383+04	46	Right	CME 55T	68	845.5	845.6	0.1 F
2	B-018-0-21	SR-14	384+07	19	Left	CME 55T	68	843.8	842.9	0.9 C
3	B-019-0-21	SR-14	385+30	53	Right	CME 55T	68	843.7	841.0	2.7 C



#	Boring	Sample	Sam De	-	_	rade pth		dard ration	НР		Pl	hysica	l Chara	acteristics		Moisture		Ohio DOT		Sulfate Content	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in
			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	2.5	4.0	2.6	4.1	8		2							26	18	A-7-6	16						
	017-0	SS-2	5.0	6.5	5.1	6.6	9		2.25							27	18	A-7-6							
	21																								
								8																	
2	В	SS-1	2.5	4.0	1.6	3.1	8		4	42	20	22	35	52	87	24	18	A-7-6	13			N ₆₀ & Mc			
	018-0	SS-2	5.0	6.5	4.1	5.6	8		2.75							22	18	A-7-6	16						
	21																								
								8																	
3	В	SS-1	1.5	3.0	-1.2	0.3	11		3.75	52	25	27	15	80	95	29	22	A-7-6	17			N ₆₀ & Mc		12"	
	019-0	SS-2	3.0	4.5	0.3	1.8	11		3.75	56	26	30	20	79	99	30	23	A-7-6	19			N ₆₀ & Mc		12"	
	21	SS-3	4.5	6.0	1.8	3.3	10		3.5							31	18	A-7-6	16			N ₆₀ & Mc			
		SS-4	6.0	7.5	3.3	4.8	10	10	4.25							28	18	A-7-6	16						



PID: 104132

County-Route-Section: CUY-14-6.93

No. of Borings: 3

Geotechnical Consultant: NEAS Inc.

Prepared By: Brendan P Andrews, PE

Date prepared: 2/13/2023

C	Chemical Stabilization Options								
320	320 Rubblize & Roll								
206	Cement Stabilization	No							
	Lime Stabilization	Option							
206	Depth	14"							

Excavate and Repl	Excavate and Replace							
Stabilization Options								
Global Geotextile								
Average(N60L):	12"							
Average(HP):	0"							
Global Geogrid								
Average(N60L):	0"							
Average(HP):	0"							

Design CBR	4
---------------	---

% Sample	es within	6 feet of subgr	ade
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ < 12	100%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	0%	1 < HP ≤ 2	13%
N ₆₀ ≥ 20	0%	HP > 2	88%
M+	50%		
Rock	0%		
Unsuitable	0%		

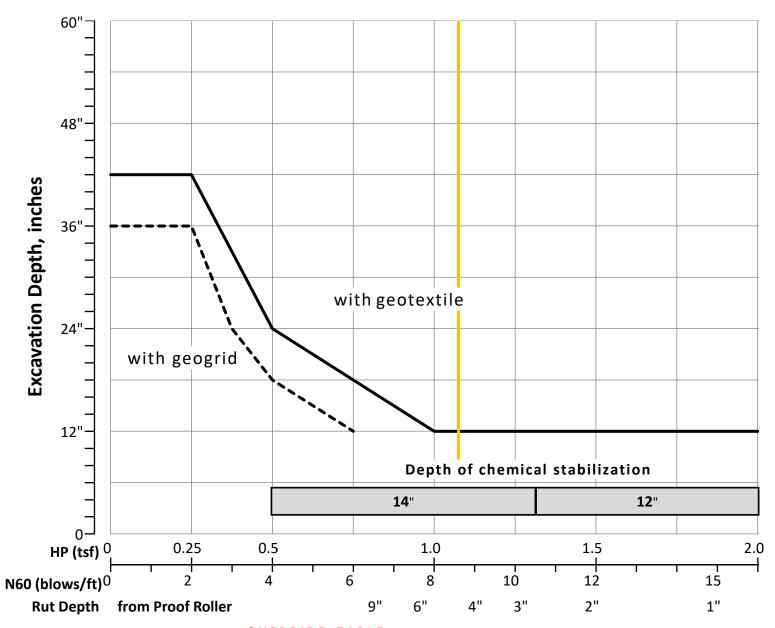
Excavate and Replace at Surface						
Average	0"					
Maximum	0"					
Minimum	0"					

% Proposed Subgrade Surface						
Unstable & Unsuitable	80%					
Unstable	80%					
Unsuitable	0%					

	N ₆₀	N _{60L}	НР	LL	PL	PI	Silt	Clay	P 200	M _c	M _{OPT}	GI
Average	9	9	3.28	50	24	26	23	70	94	27	19	16
Maximum	11	10	4.25	56	26	30	35	80	99	31	23	19
Minimum	8	8	2.00	42	20	22	15	52	87	22	18	13

	Classification Counts by Sample																		
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
Percent	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0%	0%	100%
% Rock Granular Cohesive	0%					0%								10	0%				100%
Surface Class Count	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
Surface Class Percent	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%	0%	0%	100%

GB1 Figure B – Subgrade Stabilization



OVERRIDE TABLE

Calculated Average	New Values	Check to Override
3.28	0.50	<u></u> НР
8.67	6.00	N60L



OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

CUY-14-6.93 104132

Henry St

(Replacement of existing Bridge CUY-14-0693)

NEAS Inc.

Prepared By: Date prepared:

Brendan P Andrews, PE

Monday, February 13, 2023

Brendan P. Andrews 1329 East Kemper Road

Suite 4104B

Cincinnati, OH 45246 513.337.9823 Ext. 701

brendan.andrews@neasinc.com

NO. OF BORINGS:

1





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Proposed Subgrade EL	Cut Fill





V. 14.5 1/18/2019

#	Boring	Sample	Sam De _l	•	_	rade pth		dard ration	НР	Physical Characteristics						Mo	Moisture		DOT	Sulfate Content	Problem		Excavate and Replace (Item 204)		Recommendation (Enter depth in
			From	То	From	То	N ₆₀	N _{60L}	(tsf)	Ц	PL	PI	% Silt	% Clay	P200	M _c	M _{OPT}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	0.0	1.5	-1.2	0.3	22			NP	Np	NP	11	3	14	5	6	A-1-b	0						
	022-0	SS-2	1.5	3.0	0.3	1.8	24									10	6	A-1-b	0						
	21	SS-3	3.0	4.5	1.8	3.3	29			Np	NP	NP	13	3	16	4	6	A-1-b	0						
		SS-4	4.5	6.0	3.3	4.8	46	22								5	6	A-1-b	0						



PID: 104132

County-Route-Section: CUY-14-6.93

No. of Borings: 1

Geotechnical Consultant: NEAS Inc.

Prepared By: Brendan P Andrews, PE

Date prepared: 2/13/2023

(Chemical Stabilization Option	ıs
320	Rubblize & Roll	Option
206	Cement Stabilization	No
	Lime Stabilization	Option
206	Depth	NA

Excavate and Repl	ace
Stabilization Option	ons
Global Geotextile	
Average(N60L):	#N/A
Average(HP):	
Global Geogrid	
Average(N60L):	#N/A
Average(HP):	

Design CBR	13
---------------	----

% Sampl	es within	6 feet of subgr	ade
N ₆₀ ≤ 5	0%	HP ≤ 0.5	0%
N ₆₀ < 12	0%	0.5 < HP ≤ 1	0%
12 ≤ N ₆₀ < 15	0%	1 < HP ≤ 2	0%
N ₆₀ ≥ 20	100%	HP > 2	0%
M+	0%		
Rock	0%		
Unsuitable	0%		

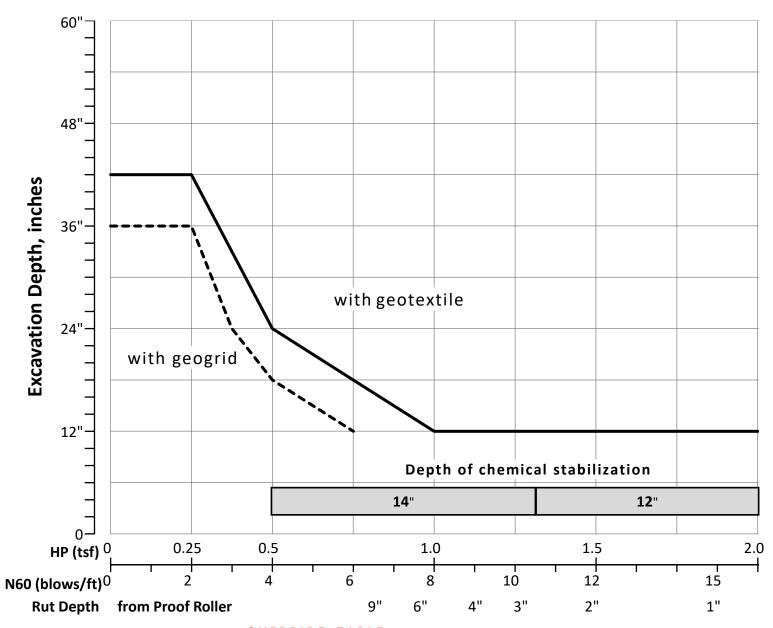
Excavate and Repl at Surface	ace
Average	0"
Maximum	0"
Minimum	0"

% Proposed Subgrade Su	rface
Unstable & Unsuitable	0%
Unstable	0%
Unsuitable	0%

	N ₆₀	N _{60L}	HP	Ш	PL	PI	Silt	Clay	P 200	M _c	M _{OPT}	GI
Average	30	22					12	3	15	6	6	0
Maximum	46	22	0.00	0	0	0	13	3	16	10	6	0
Minimum	22	22	0.00	0	0	0	11	3	14	4	6	0

	Classification Counts by Sample																		
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4
Percent	0%	0%	100%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%			-		100%	-							0	%				100%
Surface Class Count	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
Surface Class Percent	0%	0%	100%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	100%

GB1 Figure B – Subgrade Stabilization



OVERRIDE TABLE

Calculated Average	New Values	Check to Override
	4.50	✓ HP
22.00	6.00	N60L



OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

CUY-14-6.93 104132

Chaincraft Road

(Replacement of existing Bridge CUY-14-0693)

NEAS Inc.

Prepared By: Date prepared:

Brendan P Andrews, PE

Monday, February 13, 2023

Brendan P. Andrews 1329 East Kemper Road

Suite 4104B

Cincinnati, OH 45246 513.337.9823 Ext. 701

brendan.andrews@neasinc.com

NO. OF BORINGS:

5





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-006-0-21	Chaincraft Rd	110+52	28	Right	CME 75T	89	812.9	812.2	0.7 C
2	B-008-0-21	Chaincraft Rd	111+61	5	Left	CME 75T	89	815.0	814.0	1.0 C
3	B-007-0-21	Chaincraft Rd	112+78	32	Right	CME 75T	89	815.2	814.9	0.4 C
4	B-009-0-21	Chaincraft Rd	113+00	23	Left	CME 55T	68	814.9	815.0	0.1 F
5	B-021-0-21	Chaincraft Rd	114+17	25	Right	CME 55X	82	816.6	815.8	0.8 C

V. 14.5

1/18/2019

#	Boring	Sample	Sam De _l	•	Subg De	rade pth	Stan Penet		НР		P	hysica	al Chara	cteristics		Mo	isture	Ohio	DOT	Sulfate Content	Proble	m	Excavate ar	-	Recommendation (Enter depth in
			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M_{c}	M _{OPT}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	2.5	4.0	1.8	3.3	12									14	8	A-3a	0						
	006-0	SS-2	5.0	6.5	4.3	5.8	6									19	8	A-3a	0						
	21	SS-3	7.5	9.0	6.8	8.3	13			NP	NP	NP	16	2	18	14	6	A-1-b							
								6																	
2	В	SS-1	2.5	4.0	1.5	3.0	27			NP	NP	NP	32	11	43	16	11	A-4a	2			Mc			
	008-0	SS-2	5.0	6.5	4.0	5.5	10									18	8	A-3a	0						
	21	SS-3	7.5	9.0	6.5	8.0	16									24	8	A-3a							
								10																	
3	В	SS-1	2.5	4.0	2.1	3.7	4									18	6	A-1-b	0						
	007-0	SS-2	5.0	6.5	4.7	6.2	3									16	6	A-1-b	0						
	21																								
								3																	
4	В	SS-1	2.5	4.0	2.6	4.1	8									20	6	A-1-b	0						
	009-0	SS-2	5.0	6.5	5.1	6.6	1			NP	NP	NP	34	10	44	29	11	A-4a							
	21	SS-3	7.5	9.0	7.6	9.1	11									28	10	A-4a							
								1																	
5	В	SS-1	2.5	4.0	1.7	3.2	11		2.5	22	15	7	29	15	44	11	10	A-4a	2			N ₆₀			
	021-0	SS-2	5.0	6.5	4.2	5.7	5		3							19	10	A-4a	8						
	21																								
								5																	



PID: 104132

County-Route-Section: CUY-14-6.93

No. of Borings: 5

Geotechnical Consultant: NEAS Inc.

Prepared By: Brendan P Andrews, PE

Date prepared: 2/13/2023

C	Chemical Stabilization Options								
320	Rubblize & Roll	No							
206	Cement Stabilization	Option							
	Lime Stabilization	No							
206	Depth	14"							

Excavate and Replace									
Stabilization Options									
Global Geotextile									
Average(N60L):	21"								
Average(HP):	0''								
Global Geogrid									
Average(N60L):	15"								
Average(HP): 0"									

Design CBR	11
---------------	----

% Samples within 6 feet of subgrade									
N ₆₀ ≤ 5	40%	HP ≤ 0.5	0%						
N ₆₀ < 12	80%	0.5 < HP ≤ 1	0%						
12 ≤ N ₆₀ < 15	10%	1 < HP ≤ 2	0%						
N ₆₀ ≥ 20	10%	HP > 2	20%						
M+	10%								
Rock	0%		·						
Unsuitable	0%		·						

Excavate and Replace at Surface							
Average	0"						
Maximum	0"						
Minimum	0"						

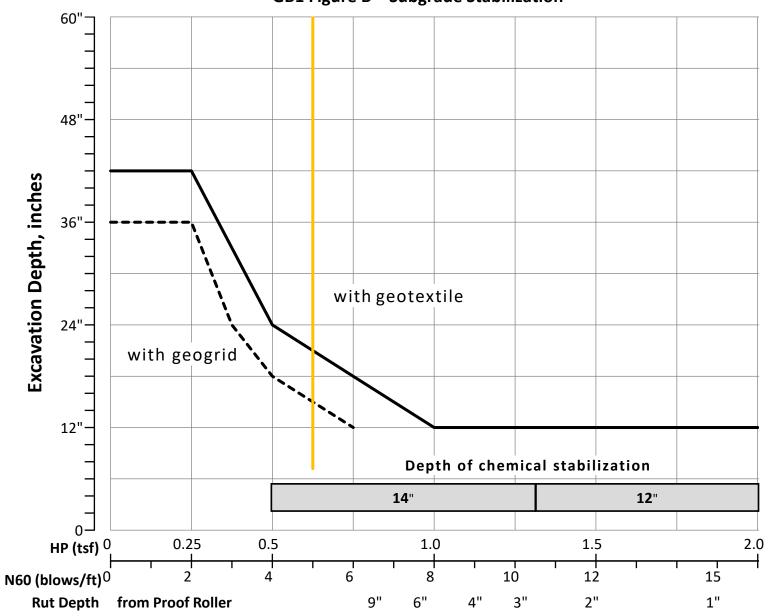
% Proposed Subgrade Surface							
Unstable & Unsuitable	40%						
Unstable	40%						
Unsuitable	0%						

	N ₆₀	N _{60L}	HP	Ш	PL	PI	Silt	Clay	P 200	M _c	M _{OPT}	GI
Average	10	5	2.75	22	15	7	28	10	37	19	8	1
Maximum	27	10	3.00	22	15	7	34	15	44	29	11	8
Minimum	1	1	2.50	22	15	7	16	2	18	11	6	0

Classification Counts by Sample																			
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	4	0	0	0	0	0	4	5	0	0	0	0	0	0	0	0	13
Percent	0%	0%	31%	0%	0%	0%	0%	0%	31%	38%	0%	0%	0%	0%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%			-	-	100%	=				0%								100%
Surface Class Count	0	0	2	0	0	0	0	0	1	2	0	0	0	0	0	0	0	0	5
Surface Class Percent	0%	0%	40%	0%	0%	0%	0%	0%	20%	40%	0%	0%	0%	0%	0%	0%	0%	0%	100%



GB1 Figure B – Subgrade Stabilization



OVERRIDE TABLE

Calculated Average	New Values	Check to Override
2.75	0.50	HP
5.00	6.00	N60L

APPENDIX D EMBANKMENT STABILITY OUTPUTS

