

CLIENT ODOT District 12	PROJECT NO.		2122-1002-0	0
PROJECT CUY-8/10-2.24/8.69 (PID 113674)				
SUBJECT Bridge No. CUY-10-0869	COMP. BY	JDA	DATE	10/27/2022
Estimated Quantity Calculations	CHECKED BY	MJL	DATE	10/28/2022

REVISION HISTORY

Stage 1 Plans:	COMP. BY	JAM	DATE	2/20/2022
	CHECKED BY	TDA	DATE	2/22/2022
Stage 3 Plans:	REVISED BY	JDA	DATE	10/27/2022
	CHECKED BY	MJL	DATE	10/28/2022

The initials and dates listed in the sheet header reflect the most recent revision of the estimated quantity calculations.

ITEM 202 - PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN

This item paid as a LUMP SUM.

Drainage Pipe Removal:					Factor 1.00	x	Length (ft) 352	x	Cost / ft \$2.00	=	\$	704.00	LS
Structural Steel Removal:					Factor 1.00	x	Weight (lb) 77,429	x	Cost / LB \$2.00	=	\$	154,858.00	LS
Expansion Joint Removal:	No.		Width (ft)		Depth (ft)		Length (ft)		Cost/ft3				
Rear Abutment Removal	1	х	2.42	х	0.75	х	52	х		=	\$	-	LS
Tower Joint Removal	5	х	1.83	х	0.75	х	52	х	\$0.00	=	\$	-	LS
Sidewalk Removal	14	х	1.83	х	0.75	х	5	х	\$0.00	=	\$	-	LS
Forward Abutment Removal	1	х	1.92	х	0.75	х	52	х	\$0.00	=	\$	-	LS
Total: Total for ITEM 202 - PORTIONS OF STRUCTURE F	REMOVED, O	VER 20		AN, A	S PER PLAN	: FME	NT AS PER		N		\$ \$	155,562.00 160,000.00	LS LS
		2/101						/ .					
From CUY-10-0869 Rebar List.xlsx									A Per Abut	Bars tment	=	490.0 245.0	LB LB
								5	Substructure	Total	=	490.0	LB
									S	Bars	=	3,481.0	LB

Superstructure Total =

200.0 LB



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ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL UF, AS PER PLAN *Item will be bid as a lump sum

Pier Tower 4:

Сар:								
L4x3 1/2x5/16 Diagonal: L6x4x3/8 Horizontal: L4x3 1/2x5/16 (Bottom of Top Cap):	No. 2 0 2	Le X X X	ength (ft) 9.22 6.00 6.00	We x x x	ight (lb/ft) 7.700 12.300 7.700	= = =	142.0 0.0 92.4	LB LB LB
Total for Cap:							234.4	LB
Columns:	Nia		L	14/-				
Diaphragm Members	NO. 1	x	1.00	x	5790.767	=	5,790.8	LB
L6"x4"x3/8" Vertical Diagonals	1	x	534.00	x	12.300	=	6,568.2	LB
Total for Columns:							12,359.0	LB
Total for Pier Tower 4:						Say:	12,594	LB
Pier Tower 5:								
Cap:	No		an ath (ft)	\M/a	abt (lb/ft)			
L4x3 1/2x5/16 Diagonal: L6x4x3/8 Horizontal: L4x3 1/2x5/16 (Bottom of Top Cap):	NO. 12 2 6	Le X X X	9.22 6.00 6.00	vve x x x	ight (id/it) 7.700 12.300 7.700	= = =	851.9 147.6 277.2	LB LB LB
Total for Cap:							1,276.7	LB
Columns:	N			14/-				
Diaphragm Members	NO. 1	X	engtn (π) 1.00	vve x	3910.533	=	3,910.5	LB
L6"x4"x3/8"	1	x	10.33	x	12.300	=	127.1	LB
Total for Columns:							4,037.6	LB
Total for Pier Tower 5:						Say:	5,315	LB
Pier Tower 6:								
Сар:	No		Length (ft)	10	eicht (lh/ft)			
L4x3 1/2x5/16 Diagonal (Vertical)	4	х	8.49	x	7.700	=	261.3	LB
L6x4x3/8 Horizontal: L4x3 1/2x5/16 (Top of Top Can):	8 10	x	6.00 9.22	X X	12.300 7 700	=	590.4 709 9	LB I B
L4x3 1/2x5/16 (Bottom of Top Cap):	4	x	6.00	x	7.700	=	184.8	LB
L4x3 1/2x5/16 (Bottom of Top Cap):	6	х	3.00	х	7.700	=	138.6	LB
Total for Cap:							1,746.5	LB
Columns:	No		longth (ft)	14	loight /lb/ft)			
Diaphragm Members	1 1	х	1.00	x	13895.833	=	13,895.8	LB
Total for Columns:							13,895.8	LB

Total for Pier Tower 6:

Say: **15,643** LB

PROJECT CUY-8/10-2.24/8.69 (PID 113674) SUBJECT Bridge No. CUY-10-0869 Estimated Quantity Calculations		001						2122-1002-00					
Estimated Quality Saterations		COM	IP. BY		JDA M.II	DA	TE <u>1</u>	<u>0/27/20</u>					
		OHL			NOL	_ 0/		0/20/20					
Pier Tower 7:													
Сар:													
	No.		Length (ft)	W	/eight (lb/ft))							
L4x3 1/2x5/16 Diagonal (vertical):	6	x	8.49	X	7.700	=	392.0	LB					
L4X3 1/2X3/10 Diagonal.	4	×	9.22	×	12 300	_	204.0						
$1/2x^{-1/2}x$	1	×	3.00	Ŷ	7 700	_	23.1	IB					
L4x3 1/2x5/16 (Bottom of Cap):	1	x	6.00	x	7.700	=	46.2	LB					
Total for Cap:							892.9	LB					
Columns:			Low with (St)		/ - : - - t / / ()								
Diaphragm Momhara	No.		Length (ft)	VV	reight (ib/ft)		01 004 4						
L6x4x3/8 Diagonal:	1	x X	485.00	x X	21321.433 12.300	=	≥1,321.4 5,965.5	LB					
Total for Columns:							27,286.9	LB					
Total for Pier Tower 7:						Say:	28,180	LB					
Pier Tower 8:													
Cap:	No		Longth (ft)	Wo	hight (lb/ft)								
4x3 1/2x5/16 Diagonal (Vertical):	2	Y	2 engin (it) 8 49	vve	7 700	=	130.7	IB					
l 6x4x3/8 Horizontal	6	x	6.00	x	12,300	=	442.8	IB					
$4x3 \frac{1}{2x5}$ (Top of Cap):	4	x	9.22	x	7 700	=	284.0	IB					
L4x3 1/2x5/16 (Bottom of Cap):	4	x	6.00	x	7.700	=	184.8	LB					
L4x3 1/2x5/16 (Bottom of Cap):	5	x	3.00	x	7.700	=	115.5	LB					
Total for Cap:							1,157.7	LB					
Columns:	.,			1.47									
Diaphragm Members	NO. 1	x	3ngth (1t) 1.00	vve x	ignt (id/it) 9.800	=	10,983.7	LB					
L6"x4"x3/8" Vertical Diagonals	1	x	289.00	x	12.300	=	3,554.7	LB					
Total for Columns:							14,538.4	LB					
Total for Pier Tower 8:						Say:	15,697	LB					
Total Steel:						Say:	77,429	LB					
Unit Cost (Ron Bauer Conversation 5/24/2021)							20	\$/LB					

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	PROJECT CUY-8/10-2.24/8.69 (PID 113674) SUBJECT Bridge No. CUY-10-0869 Estimated Quantity Calculations	COMP. BY CHECKED BY	JDA MJL	DA [·]	TE <u>1</u> TE <u>1</u>	0/27/20: 0/28/20:		
ITEM 514 - SURFACE PREPAR ITEM 514 - FIELD PAINTING O ITEM 514 - FIELD PAINTING S ITEM 514 - FIELD PAINTING S *Quantity will be lump sum. Calc *Increase area by 25% to accou	RATION OF EXISTING STRUCTURAL STEEL DF EXISTING STRUCTURAL STEEL, PRIME COAT STRUCTURAL STEEL, INTERMEDIATE COAT STRUCTURAL STEEL, FINISH COAT culate rough areas to determine cost. Refine at Stage 3 int for misc plates and angles.							
Total: Increase 25%:				= =	77,429 19,357	LB LB		
Total for Pier Towers:				Total:	96,786	LB		
Total for ITEM 514 - SURFACE Total for ITEM 514 - FIELD PA Total for ITEM 514 - FIELD PA Total for ITEM 514 - FIELD PA	E PREPARATION OF EXISTING STRUCTURAL STEEL: INTING OF EXISTING STRUCTURAL STEEL, PRIME COAT: INTING STRUCTURAL STEEL, INTERMEDIATE COAT: INTING STRUCTURAL STEEL, FINISH COAT:				96,786 96,786 96,786 96,786	_LB _LB _LB _LB		
ITEM 514 - GRINDING FINS, TI *Average cap height = 6'	EARS, SLIVERS ON EXISTING STRUCTURAL STEEL							
Total for Pier Towers:	Per 2020 BDM, Section 404.1.11, 1 minute per 1' of	beam/girder to be coated.		Say:	60	_MNHI		
Total for ITEM 514 - GRINDING	G FINS, TEARS, SLIVERS ON EXISTING STRUCTURAL STEEL:			-	60	MNHI		
TEM 514 - FINAL INSPECTION Assume one each for each cap	<u>N REPAIR</u> o and column painted		No.					
Tower Caps: Tower Columns, 1 each per co	olumn painted):		5 10	= =	5 10	EACI EACI		
Total for Pier Towers:				Say:	15	EAC		
Total for ITEM 514 - FINAL INS	SPECTION REPAIR:			-	15	_EACH		
ITEM 516 - STRIP SEAL EXPA	INSION JOINT ANCHORED WITH ELASTOMERIC CONCRETE							
Superstructure:								
Rear Abutment Joint:				=	62.00	FT		
Pier 4 Joint:				=	62.00	FT		
Pier 5 Joint:				=	62.00	FT		
Pier 6 Joint:				=	62.00	FT		
Pier 7 Joint:				=	62.00	FT		
Pier 8 Joint:				=	62.00	FT		
i orward Abutment Joint.				-	02.00			
Total:					434.00	FT		
Total for Superstructure:				Say:	434	_FT		
Total for ITEM 516 - STRIP SE	AL EXPANSION JOINT ANCHORED WITH ELASTOMERIC CON	ICRETE:		-	434	_FT		
ITEM 518 - 8" PIPE DOWNSPC	DUT, INCLUDING SPECIALS							
Superstructure:								
Locations From Downspout Re	epairs Table:			=	10.00	FT		
Subtotal:					10.00	FT		
For preliminary calculations, a	dd 20% contingency (and refine calculations at Stage 3):			=	2.00	FT		
					12.00	FT		
Total:								
Total: Total for Superstructure:				Say:	12	_FT		



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ITEM 518 - STRUCTURE DRAINAGE, MISC .: 10" GALVANIZED STEEL PIPE, INCLUDING SPECIALS

Superstructure:

Locations From Downspout Repairs Table:	=	280.00	FT
Pier 3, South Side, Upper:	=	10.00	FT
Span 5 Column 10, North Side, Upper:	=	10.00	FT
Span 5 Column 11 - Pier 5, South Side, Lower:	=	10.00	FT
Pier 5 - Span 6 Column 1, North Side, Upper:	=	10.00	FT
Span 7 Column 12 - Pier 7, North Side, Upper:	=	10.00	FT
Span 8 Column 11, South Side, Upper:	=	10.00	FT
Subtotal:		340.00	FT
Total:		340.00	FT
Total for Superstructure:	Say:	340	_FT
Total for ITEM 518 - STRUCTURE DRAINAGE, MISC.: 10" GALVANIZED STEEL PIPE, INCLUDING SPECIALS:		340	FT

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ITEM 518 - STRUCTURE DRAINAGE, MISC .: PIER TOWER COVERINGS

This item paid as an each quantity for each tower.	Say:	5	EACH
Sheet Metal:	1		
Exterior Plate Y is 3.58 x 7.75 Interior Plate Y is 4.5 x 7.75 Plate X is 2 x 2 0.25" thick plates 2-Ext. Plate Ys, 6-Int. Plate Ys, and 7-Plate Xs per pier		PI 1615[8	
Number of Exterior Plate Y = 10 Each Number of Interior Plate Y = 30 Each Number of Plate X = 35 Each From Sheet metal Website link to the right unit cost: Plat	і л	Plate X	іл: п]
Plate Y Unit Cost = \$ 1,000 Plate Y Unit Cost = \$ 1,000 Plate Y Unit Cost = \$ 1,000 Note I increased these as they were Plate X Unit Cost = \$ 200 surprisely cheap compared to the steel MetalsDepot® - 6061 Aluminum Sheet 6	061 Alumi	num Plate	
Increase cost by a factor of 1.5 for fabrication (bending & holes) Installation cost: Assume a three man crew can install 0.5 piers per day = \$ 12,000 (assuming a \$50/hr Labor Rate) Total Shart Metal cost = \$ 22,000 \$ 250/hr Labor Rate)			
$10 \text{ tal Sheet Metal Cost} = \frac{5 82,500}{2}$			
2= Number of Exterior Plate Y\$ 2,000= Cost of Exterior Plate Y per pier6= Number of Interior Plate Y\$ 6,000= Cost of Interior Plate Y per pier7= Number of Plate Xs\$ 1,400= Cost of Plate X per pier2= Days to Install Plates\$ 2,400= Installation Cost per pier \$ 11,800per pier			
/ibration and Sound Damping: <u>https://acousticalsolutions.com/product/vibstop-vibration-damping-sheet/?utm_source=bing&utm_medium=cpc&utm_campaign=DSA - Bing&utm_content=All%20Pages&utm_term=acousticalsolutions&msclkid=8d86e8164d2c1e1328c1887d1b23b591 Apply to both Plate Y and Plate X Total area = 1464.0 sf </u>	lying on ot	and motol	
Assume unit cost from link to the right = 5 0.00 /st Assume 20% increase for installation which should be simply pealing and stic	King on sr	ieet metal	
Total Vibration and Sound Damping Cost = <u>\$ 8,784</u>			
Total Cost = \$ 91,284 Increase 20% for incidentals (bolts, connections, etc)	Say:	\$ 22,000	EACH

ITEM 518 - STRUCTURE DRAINAGE, MISC.: BRIDGE DRAINAGE SYSTEM CLEANING

This item paid as a LUMP SUM.