# INTEROFFICE COMMUNICATION

TO: Erika J. Kenzig, PE, PM, PID 122289

FROM: Kyle J. Dohlen, PE, D12, District Geotechnical Engineer

DATE: October 22, 2024

SUBJECT: Geotechnical Pavement Recommendations - PID 122289

Ms. Kenzig,

## **Executive Summary**

This project will consist of the full depth pavement replacement of the existing concrete pavement the Ridge Road entrance ramps to IR 480 EB and IR 480 WB. The proposed pavement for the ramps will be unreinforced concrete. At the time of this report, it is our understanding that there will be no profile changes to the ramps.

#### Ramp R-2 (Ridge to 480 WB) Encountered Subsurface materials

Pavement comprised of reinforced concrete was encountered in three (3) borings ranging in thickness from 9.25 to 9.5 inches. Asphalt pavement was encountered in one (1) boring (B-9) and the pavement thickness was noted as 17.75 inches. Underlying the pavement, aggregate base was encountered in all the borings ranging from 5.0 to 12.0 inches. Cohesive soils noted as damp, very stiff Silt and Clay (A-6a) were encountered in one boring (B-10). Beneath the aggregate base and cohesive soils, bedrock was encountered in all four borings. The bedrock was described as red, highly to severely weathered, very weak, Shale. The shale was encountered between 1.3 and 2.5 feet below the top of pavement. Groundwater was not encountered in any of the borings.

#### Ramp R-3 (Ridge Road to 480 EB) Encountered Subsurface materials

Asphalt pavement was encountered in one boring, B-8 and the pavement thickness was noted as 9 inches. Below the asphalt pavement and in the remaining three (3) borings concrete pavement was encountered, the pavement varied in thickness from 8.0 to 9.5 inches. Aggregate base was encountered in all three borings and ranged in thickness from 6.0 to 18.0 inches. Below the pavement and aggregate base cohesive soils comprised of damp, stiff to very stiff Silt and Clay (A-6a) were encountered in two borings, B-7 and B-8. Bedrock was encountered in 2 borings, B-6 and B-7, the bedrock was described as moderately to highly weathered, red and gray Shale. In boring B-6, bedrock was encountered beneath the aggregate base at depth of 1.2' below the top of pavement. In boring B-7, bedrock was encountered at depth of 2.8' below the top of the pavement. Groundwater was not encountered in any of the borings.

The subgrade analysis spreadsheets for this project indicated that unstable soils comprised high moisture soils at Ramp R-3 were encountered from approximately Sta. 719+50 to Sta. 723+50. Unsuitable soils comprised of shallow Shale bedrock was encountered in Ramp R-2 from Sta. 710+00 to the Ridge Road intersection and in Ramp R-3 from the Ridge Road intersection to approximately Sta. 719+50. The shale bedrock will be required to be removed to a depth of 6 inches below the final subgrade elevation.



#### Introduction

This project will consist of the full depth pavement replacement of the existing concrete pavement along the Ridge Road entrance ramps to IR 480 EB and IR 480 WB. The proposed pavement for the ramps will be unreinforced concrete. At the time of this report, it is our understanding that there will be no profile changes to the ramps. The anticipated total length of work for the ramps is as follows: Ramp R-2 approximately 1000' and Ramp R-3 approximately 825'

#### Geology

This project site is located within the glaciated portion of the state and the Erie Lake Plain physiographic region. This region is characterized as very-low relief Ice Age lake basin separated from modern Lake Erie by shoreline cliffs with major streams in deep gorges. The geology for this region can be characterized by Pleistocene-age lacustrine sand, silt, clay and wave-planed till over Devonian and Mississippian-age shales and sandstones.

#### Reconnaissance

A field reconnaissance was performed on October 8<sup>th</sup>, 2024 by ODOT (Kyle Dohlen). The following observations were noted during the reconnaissance.

## Ridge to 480 WB (Ramp (R-2)

Existing concrete pavement exhibited numerous open joints which have been filled with asphalt to improve rideability. Adjacent to the traffic island in the northern entrance lane pavement was showing mid-slab cracking near a catch basin. It appears that the cracking was inline with a conduit connecting two catch basins which indicates either material loss over the conduit or damage to the conduit. Overall pavement condition for the ramp is fair to poor with several slabs previously being repaired.

## Ridge to 480 EB (Ramp R-3)

Paving overlay performed under a purchase order in calendar year 2024, approximately 1.5" of an asphalt overlay was placed. At the time of the reconnaissance cracks were telegraphing to the surface from the concrete pavement. The asphalt pavement was in good condition. Near the ramp tie in to IR-480 EB, the pavement condition changed to fair, numerous sealed cracks were evident and the outside shoulder pavement was beginning to ravel.

#### **Explorations**

#### Historic borings

Borings were performed in 1978 for the original construction of the IR-480 and Ridge Road intersection. A total of three (3) borings were performed along Ramp R-2 and a total of five (5) borings were performed along Ramp R-3.

Ramp R-2 was built in a cut section with all three borings terminating in weathered shale bedrock. At the proposed profile of the ramp, soils comprised of Gravel with Sand and Silt (A-2-4) and Silt and Clay (A-6a) and shale bedrock were encountered.

Ramp R-3 was built in a cut and fill section, the three (3) borings in the cut terminated in shale bedrock or auger refusal with no note about encountered material, the two (2) borings in the fill section terminated in shale bedrock. At the proposed profile of the ramp, soils comprised of Clay (A-7-6) and shale bedrock were encountered. The boring performed at STA 721+00 encountered auger refusal at a depth of approximately 8.5 feet below the existing ground surface but there is no note on the field log or soil profile sheet regarding the material encountered at this depth.



#### **Project Borings**

Explorations for this project were performed by the District geotechnical sub-surface exploration consultant, SME. The project borings at the Ridge Road entrance ramps under were performed under PID 119642. A total of sixteen (16) borings were performed for this exploration, with seven (7) borings being used for this report. Borings performed on Ramp R-3 (Ridge to 480 EB) are labelled as B-006-0-23 through B-008-0-23 and borings performed on Ramp R-2 (Ridge to 480 WB) are labeled B-009-0-23 through B-012-0-23. These borings will subsequently be referred to as B-6 through B-12. Explorations at these locations were performed between July 27<sup>th</sup>, 2023 and August 1<sup>st</sup>, 2023. The borings were performed using a CME 55 truck drill rig using an automatic hammer with an energy transfer ratio of 77% which was calibrated on 8/5/2022. Boring termination depths ranged from 6.03' to 9.0' below the existing pavement subgrade, if encountered.

## **Findings**

## Ramp R-2 (Ridge to 480 WB) Subsurface Conditions

Pavement comprised of reinforced concrete was encountered in three (3) borings ranging in thickness from 9.25 to 9.5 inches. Asphalt pavement was encountered in one (1) boring (B-9) and the pavement thickness was noted as 17.75 inches. Underlying the pavement, aggregate base was encountered in all the borings ranging from 5.0 to 12.0 inches. Cohesive soils noted as damp, very stiff Silt and Clay (A-6a) were encountered in one boring (B-10). Beneath the aggregate base and cohesive soils, bedrock was encountered in all four borings. The bedrock was described as red, highly to severely weathered, very weak, Shale. The shale was encountered between 1.3 and 2.5 feet below the top of pavement. Groundwater was not encountered in any of the borings.

#### Ramp R-3 (Ridge Road to 480 EB) Subsurface Conditions

Asphalt pavement was encountered in one boring, B-8 and the pavement thickness was noted at 9 inches. Below the asphalt pavement and in the remaining three (3) borings, concrete pavement was encountered, the concrete pavement varied in thickness from 8.0 to 9.5 inches. Aggregate base was encountered in all three borings and ranged in thickness from 6.0 to 18.0 inches. Below the pavement and aggregate base cohesive soils comprised of damp, stiff to very stiff Silt and Clay (A-6a) were encountered in two borings, B-7 and B-8. Bedrock was encountered in 2 borings, B-6 and B-7, the bedrock was described as moderately to highly weathered, red and gray Shale. In boring B-6, bedrock was encountered beneath the aggregate base at depth of 1.2' below the top of pavement. In boring B-7, bedrock was encountered at depth of 2.8' below the top of the pavement. Groundwater was not encountered in any of the borings.

#### Sulfate Testing

Sulfate testing was performed on borings B-6 through B-12. Sulfate test results ranged from 189 ppm to 822 ppm.

#### **Encountered Pavement**

A summary of the encountered pavements are identified in Table 1 - Encountered Pavement



Table 1 – Encountered Pavement

Boring Location	Asphalt, in.	Concrete, in.	Aggregate Base, in.
B-006-0-23	*	8	6
B-007-0-23	*	9.5	6
B-008-0-23	9	9	18
B-009-0-23	17.75	*	12
B-010-0-23	*	9.25	6
B-011-0-23	*	9.5	5
B-012-0-23	*	9.5	10

<sup>\*</sup> Not encountered

#### Recommendations

#### **Pavement Support Parameters**

The ODOT subgrade analysis spreadsheet has provided the following CBR and Modulus of Subgrade reaction values for this project.

Table 2 – Pavement Support Parameters

Alignment	CBR, %	Mr, psi
R-2 - Ridge Road to 480 WB	12	14,400
R-3 - Ridge Road to 480 EB	9	10,800

#### **Unsuitable Soils**

Unsuitable soils are defined in the ODOT GDM as soils or bedrock of the following classifications: A-4b, A-2-5, A-5, A-7-5, A-8a, A-8b, soils with a Liquid Limit greater than 65 and shallow Rock, Shale or Coal. Only shallow bedrock was encountered during the explorations for this project. When shallow bedrock is encountered, Section 610.5 of the GDM states that 6 inches of the material is to be removed from below the final subgrade elevation.

Shallow bedrock was encountered at or near the subgrade elevation along both the R-2 and R-3 ramps at Ridge Road. Removal limits for the encountered bedrock are identified in Table 2 – Subgrade Remediation. Bedrock removal is noted in the Subgrade Analysis document for Ramp R-2 starting at boring B-9. The pavement encountered in the boring is approximately 17.75 inches thick with a 12 inch aggregate base. With the proposed replacement pavement, bedrock should not be encountered in any excavation for this boring.

Within the limits of the rock undercuts, quantities for Item 605 – Rock Cut Underdrains should be included in the plans.

#### **Unstable Soils**

Unstable soils were identified at the R-3 ramp and the subgrade soils were noted to have a high moisture content. These soils should be proof rolled to identify unstable subgrade soils. A contingency quantity of Item 203 Excavation and Item 203 Embankment should be included in the plans. Removal limits for the encountered unstable soils are identified in Table 2 – Subgrade Remediation.



Table 3 – Subgrade Remediation

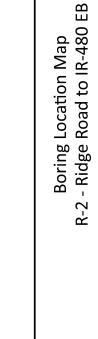
Alignment	Begin Station	End Station	Subgrade Recommendation	Cause
R-2	710+00	End of Ramp Work	6" Rock Undercut	Unsuitable
R-3	Beginning of Ramp Work	719+50	6" Rock Undercut	Unsuitable
R-3	719+50	723+50	Proof Roll	Unstable Soils



Appendix



RIDGE ROAD ₺



B-008-0-23

∕– IR-480 €

RAMP R-2 B

721

B-007-0-23

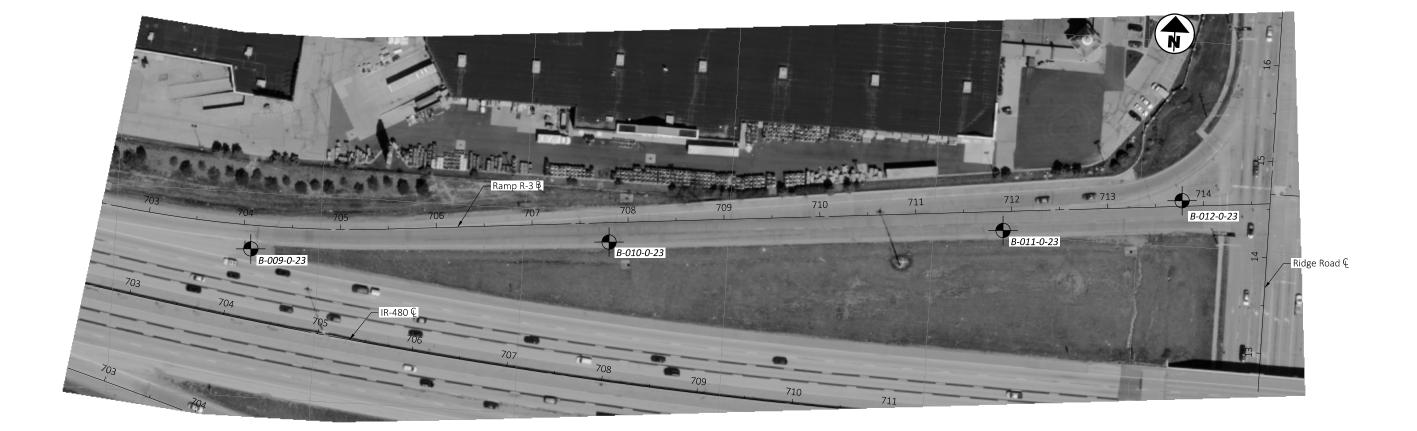
721

720

KJD

122289

Boring Location Map Ramp R-3 - Ridge Road to IR 480 WB



DESIGNER
KJD
REVIEWER

PROJECT ID	)
122	298
SHEET	TOTAL
2	2

Boring Logs



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ABANDONMENT METHODS, MATERIALS, QUANTITIES: ASPHALT PATCH; BACKFILLED WITH AUGER CUTTINGS

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ABANDONMENT METHODS, MATERIALS, QUANTITIES: ASPHALT PATCH; BACKFILLED WITH AUGER CUTTINGS

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EOB EOB	₹ 769 7	.   _   _	-	-	_	-	Rock (V	)   -	7
	SI FOB EOB						,	<u> </u>	
	SHALE GRAY, HIGHLY WEATHERED, VERY WEAK TO WEAK.  769.7  EOB  769.7  SHALE GRAY, HIGHLY WEATHERED, VERY WEAK TO WEAK.  770 SS-1B								

TYPE:         ROADWAY         S           PID:         119642         SFN:         C	DRILLING FIRM / OPERATOR SAMPLING FIRM / LOGGER: DRILLING METHOD: SAMPLING METHOD:		HAM CALI	MER: BRATI	CME 55 CME A ON DATE: ATIO (%):	MOTUA 8.		;	STATALIC	MME VATIO	ENT: ON: _		.5 (M	CL SL)	EOB:		- 1	
MATERIAL DESCRIPTION AND NOTES	ELEV. 775.5	DEPTHS SPT/	NI		SAMPLE ID		(		DATIO FS		)	ATT	ERB	ERG		ODOT CLASS (GI)	SO4 ppm	BACK FILL
9.5" CONCRETE	774.7			(70)	טו	(tSI)	GIV	00	10	OI .	OL	LL	1.5		WO			
10" AGGREGATE BASE	773.9	— 1 —																
3 SHALE, RED, HIGHLY WEATHERED, VERY W	VEAK. 772.4	- 2 - 8 - 15 - 3 -	42	78	SS-1	-	-	-	-	1	-	-	-	-	-	Rock (V)	595	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
SHALE, GRAY, HIGHLY WEATHERED, VERY TO WEAK.	WEAK	- - 4 - 16 25 40	83	100	SS-2	-	-	-	-	-	-	-	-	-	-	Rock (V)	-	17 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		- 5 - 20 36 50/3	-	100	SS-3	-	-	-	-	-	-	-	-	-	-	Rock (V)	-	
	768.7	18 50/3"	-	100	SS-4	-	-	-	-	-	-	-	-	-	-	Rock (V)	-	
SHALE, GRAY, HIGHLY WEATHERED, VERY TO WEAK.																		

Subgrade Analysis Reports





## OHIO DEPARTMENT OF TRANSPORTATION

## OFFICE OF GEOTECHNICAL ENGINEERING

# PLAN SUBGRADES Geotechnical Design Manual Section 600

Instructions: Enter data in the shaded cells only. (Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

CUY-480-7.28/VAR Ramps 122289

# Full Depth Replacement of the Ridge to CUY-480 WB Ramp

## **ODOT - District 12**

Prepared By: Kyl

Kyle J. Dohlen, PE

Date prepared:

Wednesday, May 1, 2024

Kyle J. Dohlen, PE

5500 Transportation Boulevard Garfield Heights, Ohio 44116

216-584-2125

Kyle.Dohlen@dot.ohio.gov

NO. OF BORINGS:

4





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-009-0-23	BL Ramp R-2	704+09	25	Rt.	CME 55 293	77	781.7	780.3	1.4 C
2	B-010-0-23	BL Ramp R-2	707+80	20	Rt.	CME 55 293	77	778.8	777.4	1.4 C
3	B-011-0-23	BL Ramp R-2	711+90	20	Rt.	CME 55 293	77	775.9	774.5	1.4 C
4	B-012-0-23	BL Ramp R-2	713+78	6	Lt.	CME 55 293	77	775.5	774.1	1.4 C

4/4/2024



#	Boring	Sample	Sam De <sub>l</sub>	-	_	rade pth		dard ration	НР		Pl	nysica	ıl Chara	cteristics		Moi	sture	Ohio	DOT	Sulfate Content	Proble	m	Excavate ar		Recommendation (Enter depth in
			From	То	From	То	N <sub>60</sub>	N <sub>60L</sub>	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M <sub>c</sub>	M <sub>OPT</sub>	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	2.5	3.5	1.1	2.1	80										0	Rock	0	286	Rock		6"		
	009-0	SS-2	4.0	5.0	2.6	3.6	68										0	Rock	0						
	23	SS-3	5.5	6.5	4.1	5.1	73										0	Rock	0						
		SS-4	7.0	7.5	5.6	6.1	100										0	Rock							
2	В	SS-1	1.3	2.8	-0.1	1.4	32		3.5	36	21	15	36	53	89	15	16	A-6a	10	600					
	010-0	SS-2	2.8	4.3	1.4	2.9	47										0	Rock	0						
	23	SS-3	4.3	5.6	2.9	4.2	100										0	Rock	0						
		SS-4	5.8	6.2	4.4	4.8	60	30									0	Rock	0						
3	В	SS-1	1.3	2.8	-0.1	1.4	36										0	Rock	0	212	Rock		6"		6" Rock Excavation
	011-0	SS-2	2.8	4.3	1.4	2.9	77										0	Rock	0						
	23	SS-3	4.3	4.8	2.9	3.4	100										0	Rock	0						
		SS-4	5.8	6.3	4.4	4.9	100										0	Rock	0						
4	В	SS-1	1.6	3.1	0.2	1.7	42										0	Rock	0	595	Rock		6''		
	012-0	SS-2	3.1	4.6	1.7	3.2	83										0	Rock	0						
	23	SS-3	4.6	5.9	3.2	4.5	100										0	Rock	0						
		SS-4	6.1	6.9	4.7	5.5	100										0	Rock	0						



**PID:** 122289

**County-Route-Section:** CUY-480-7.28/VAR Ramps

No. of Borings: 4

**Geotechnical Consultant:** ODOT - District 12

Prepared By: Kyle J. Dohlen, PE

**Date prepared:** 5/1/2024

(	Chemical Stabilization Option	iS								
320	Rubblize & Roll	Option								
206	206 Cement Stabilization									
	Lime Stabilization	No								
206	Depth	NA								

Excavate and Repl	ace
Stabilization Option	ons
<b>Global Geotextile</b>	
Average(N60L):	0''
Average(HP):	0"
Global Geogrid	
Average(N60L):	0''
Average(HP):	0''

Design CBR	12
---------------	----

% Samples within 3 feet of subgrade													
0%	HP ≤ 0.5	0%											
0%	0.5 < HP ≤ 1	0%											
0%	1 < HP ≤ 2	0%											
63%	HP > 2	6%											
0%													
88%													
0%		·											
	0% 0% 0% 63% 0% 88%	0% HP ≤ 0.5 0% 0.5 < HP ≤ 1 0% 1 < HP ≤ 2 63% HP > 2 0% 88%											

Excavate and Repl at Surface	ace
Average	0''
Maximum	0''
Minimum	0"

% Proposed Subgrade Su	ırface
Unstable & Unsuitable	38%
Unstable	0%
Unsuitable (Soil & Rock)	38%

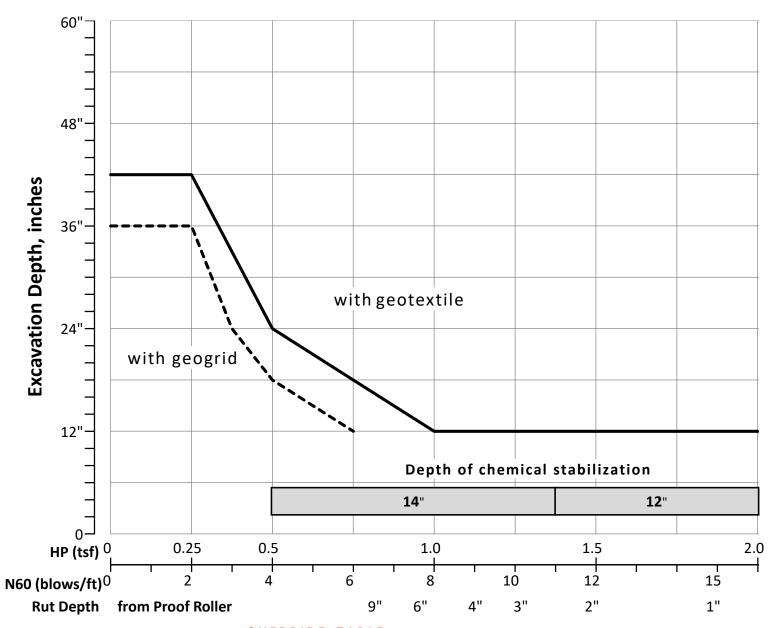
	N <sub>60</sub>	N <sub>60L</sub>	НР	LL	PL	PI	Silt	Clay	P 200	$M_{c}$	M <sub>OPT</sub>	GI
Average	75	30	3.50	36	21	15	36	53	89	15	1	1
Maximum	100	30	3.50	36	21	15	36	53	89	15	16	10
Minimum	32	30	3.50	36	21	15	36	53	89	15	0	0

					Class	ificat	ion C	Classification Counts by Sample														
ODOT Class	UCF	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals		
Count	0	15	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	16		
Percent	0%	94%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	6%	0%	0%	0%	0%	0%	100%		
% Rock Granular Cohesive	0%	94%					0%								6	%				100%		
Surface Class Count	0	7	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	8		
Surface Class Percent	0%	88%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	13%	0%	0%	0%	0%	0%	100%		

4/4/2024



Fig. 600-1 – Subgrade Stabilization



# OVERRIDE TABLE

Calculated Average	New Values	Check to Override
3.50	0.50	HР
30.00	6.00	N60L

Average HP Average N<sub>60L</sub>



## OHIO DEPARTMENT OF TRANSPORTATION

## OFFICE OF GEOTECHNICAL ENGINEERING

# PLAN SUBGRADES Geotechnical Design Manual Section 600

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CUY-480-7.28/VAR Ramps 122289

# Full Depth Replacement of Ridge to CUY-480 EB Ramp

## **ODOT - District 12**

Prepared By: Kyle J. Dohlen, PE

Date prepared: Thursday, May 2, 2024

Kyle J. Dohlen, PE 5500 Transportation Boulevard Garfield Heights, Ohio 44116

216-584-2125

Kyle.Dohlen@dot.ohio.gov

NO. OF BORINGS:

3





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring	Proposed Subgrade EL	Cut Fill
1	B-006-0-23	BL Ramp R-3	717+50	6	Rt.	CME 55 Truck 293	77	771.8	770.4	1.4 C
2	B-007-0-23	BL Ramp R-3	721+32	4	Rt.	CME 55 Truck 293	77	781.1	779.7	1.4 C
3	B-008-0-23	BL Ramp R-3	725+40	31	Lt.	CME 55 Truck 293	77	790.6	789.2	1.4 C

4/4/2024



#	Boring	Sample	Sam De <sub>l</sub>	•	Subg De	rade pth		dard ration	НР		Pł	nysica	al Chara	cteristics		Mo	isture	Ohio	DOT	Sulfate Content	Problem		Excavate ar	•	Recommendation (Enter depth in	
-			From	То	From	То	N <sub>60</sub>	N <sub>60L</sub>	(tsf)	LL	PL	PI	% Silt	% Clay	P200	M <sub>c</sub>	M <sub>OPT</sub>	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)	
1	В	SS-1	1.2	2.7	-0.2	1.3	87										0	Rock	0		Rock		6"			
	006-0	SS-2	2.7	2.9	1.3	1.5	100										0	Rock	0							
	23	SS-3	4.2	4.6	2.8	3.2	100										0	Rock	0							
		SS-4	5.7	6.0	4.3	4.6	100										0	Rock	0							
2	В	SS-1	1.3	2.8	-0.1	1.4	15		3.25	33	21	12	32	45	77	19	16	A-6a	9	337		Mc				
	007-0	SS-2	2.8	4.3	1.4	2.9	53										0	Rock	0							
	23	SS-3	4.3	5.8	2.9	4.4	39										0	Rock	0							
		SS-4	5.8	6.7	4.4	5.3	100	15									0	Rock	0							
3	В	SS-1	3.0	4.5	1.6	3.1	14		2	33	20	13	27	39	66	15	15	A-6a	7	822						
	008-0	SS-2	4.5	6.0	3.1	4.6	15		2.5							17	14	A-6a	10							
	23	SS-3	6.0	7.5	4.6	6.1	6		1.75	36	21	15	30	42	72	15	16	A-6a	9							
		SS-4	7.5	9.0	6.1	7.6	9	6	2.5							15	14	A-6a								



**PID:** 122289

**County-Route-Section:** CUY-480-7.28/VAR Ramps

No. of Borings: 3

**Geotechnical Consultant:** ODOT - District 12 **Prepared By:** Kyle J. Dohlen, PE

**Date prepared:** 5/2/2024

C	<b>Chemical Stabilization Options</b>									
320	320 Rubblize & Roll No									
206	206 Cement Stabilization									
	Lime Stabilization	No								
206	Depth	14"								

Excavate and Replace							
<b>Stabilization Options</b>							
Global Geotextile							
Average(N60L):	12"						
Average(HP):	0"						
Global Geogrid							
Average(N60L):	0''						
Average(HP): 0"							

Design CBR	9
---------------	---

% Sample	% Samples within 3 feet of subgrade								
N <sub>60</sub> ≤ 5	0%	HP ≤ 0.5	0%						
N <sub>60</sub> < 12	0%	0.5 < HP ≤ 1	0%						
12 ≤ N <sub>60</sub> < 15	8%	1 < HP ≤ 2	8%						
N <sub>60</sub> ≥ 20	42%	HP > 2	17%						
M+	8%								
Rock	67%								
<b>Unsuitable Soil</b>	0%								

Excavate and Replace at Surface							
Average	0''						
Maximum	0''						
Minimum	0"						

% Proposed Subgrade Su	% Proposed Subgrade Surface							
Unstable & Unsuitable	33%							
Unstable	17%							
Unsuitable (Soil & Rock)	17%							

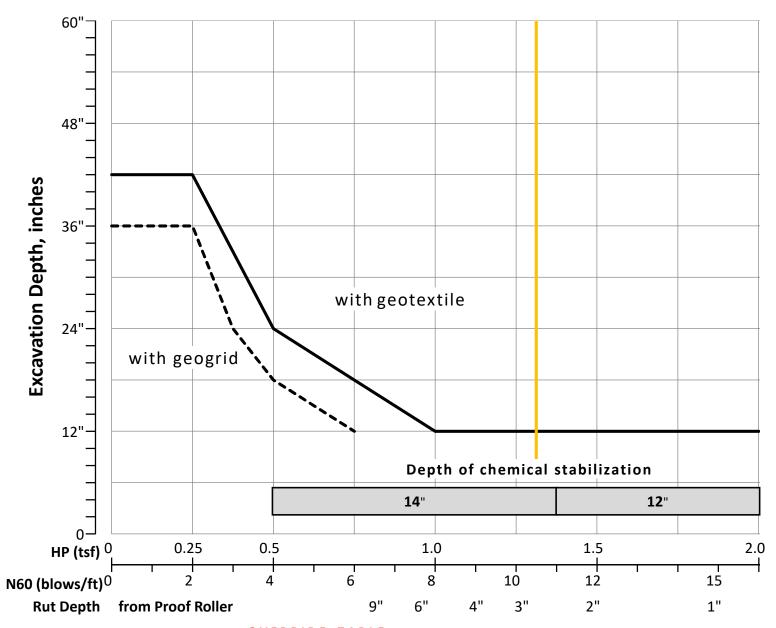
	N <sub>60</sub>	N <sub>60L</sub>	HP	LL	PL	PI	Silt	Clay	P 200	$M_{c}$	M <sub>OPT</sub>	GI
Average	53	11	2.40	34	21	13	30	42	72	16	6	3
Maximum	100	15	3.25	36	21	15	32	45	77	19	16	10
Minimum	6	6	1.75	33	20	12	27	39	66	15	0	0

Classification Counts by Sample																				
ODOT Class	UCF	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	7	0	0	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0	12
Percent	0%	58%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	42%	0%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	0%	58%		0%								42%								100%
Surface Class Count	0	4	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	6
Surface Class Percent	0%	67%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	33%	0%	0%	0%	0%	0%	100%

4/4/2024



Fig. 600-1 – Subgrade Stabilization



# OVERRIDE TABLE

Calculated Average	New Values	Check to Override
2.40	0.50	HР
10.50	6.00	N60L

Average HP Average N<sub>60L</sub>