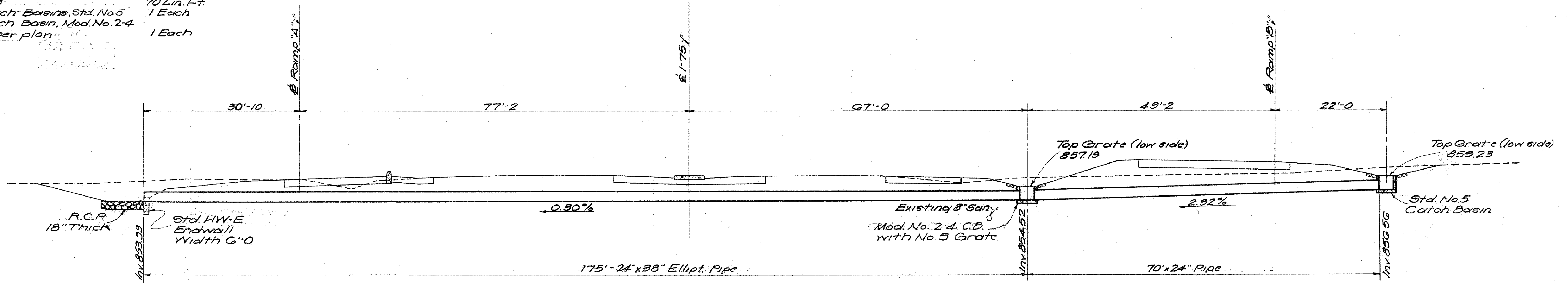


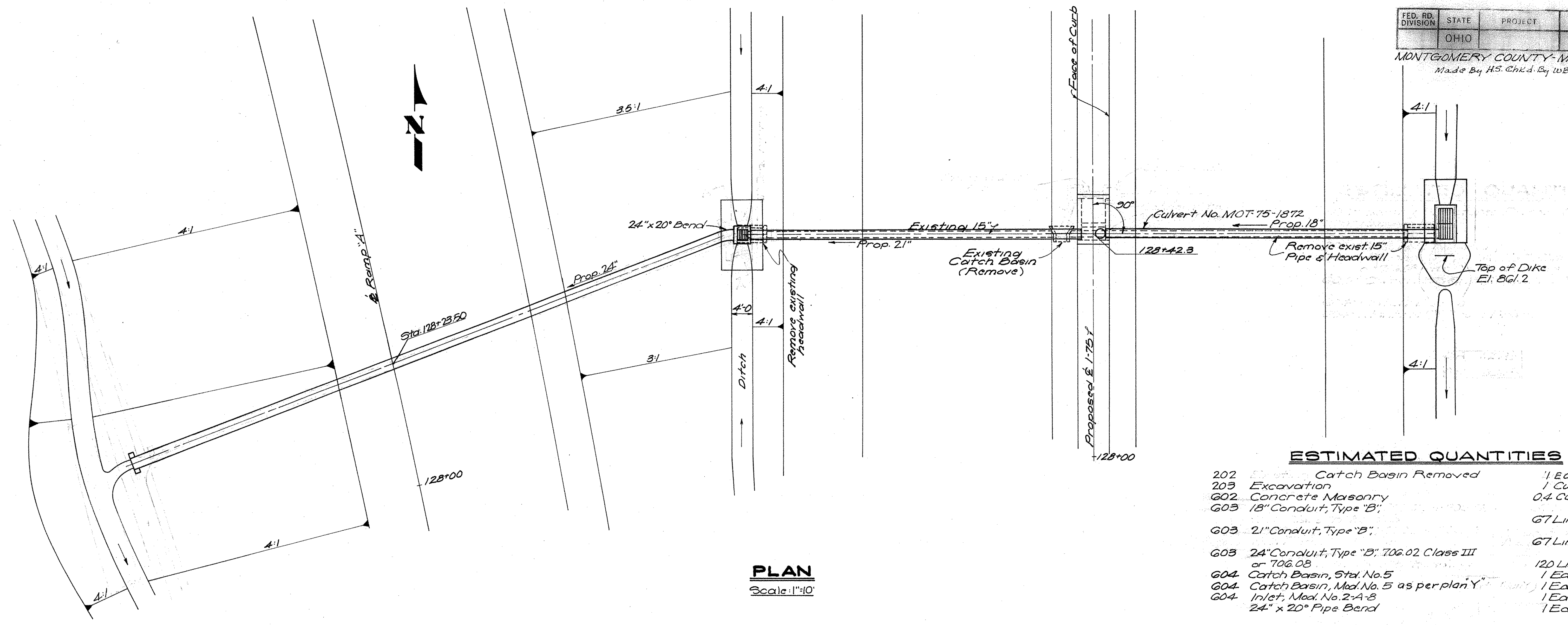
ESTIMATED QUANTITIES

- 203 - Excavation 1 Cu. Yd.
- G01 - Rock Channel 8.4 Cu. Yds.
- Protection Type B 0.7 Cu. Yds.
- G02 - Concrete Masonry 175 Lin. Ft.
- G03 - 24" x 38" Conduit, Type "A", 70604 Class HE II 70 Lin. Ft.
- G03 - 24" Conduit, Type "A", 70602 or 70603 1 Each
- G04 - Catch Basins, Std. No. 5 1 Each
- G04 - Catch Basin, Mod. No. 2-4 as per plan 1 Each

PLAN
Scale 1" = 10'

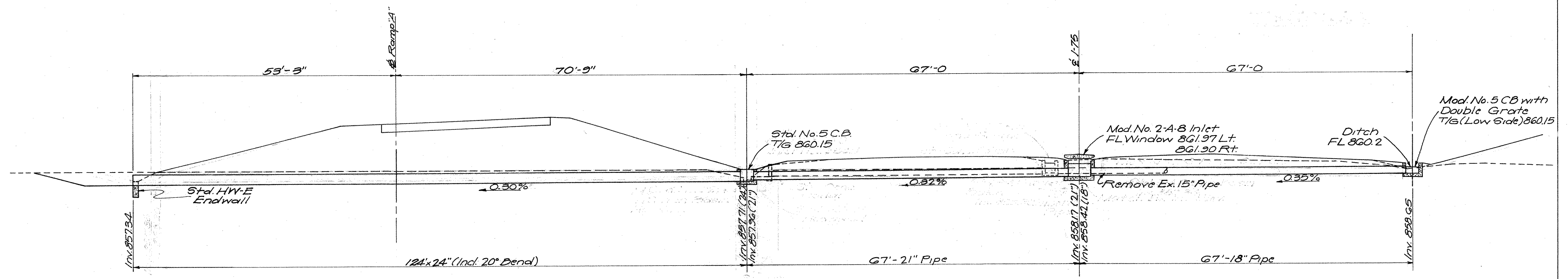


SECTION ON E PIPE

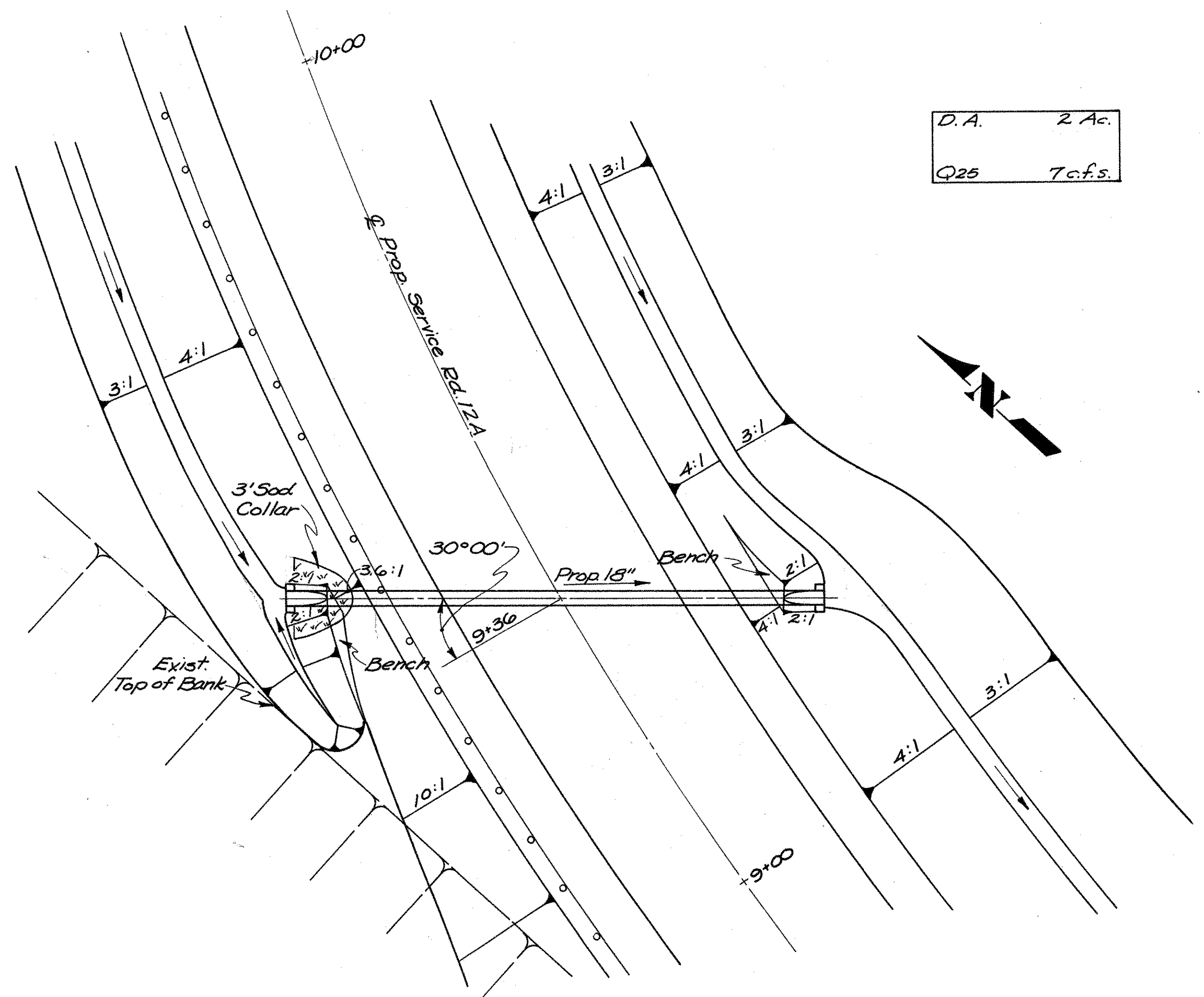


ESTIMATED QUANTITIES

202	Catch Basin Removed	1 Each
203	Excavation	1 Cu. Yd.
602	Concrete Masonry	0.4 Cu. Yd.
603	18" Conduit, Type "B"	67 Lin. Ft.
603	21" Conduit, Type "B"	67 Lin. Ft.
603	24" Conduit, Type "B", 706.02 Class III or 706.08	120 Lin. Ft.
604	Catch Basin, Std. No. 5	1 Each
604	Catch Basin, Mod. No. 5 as per plan Y	1 Each
604	Inlet, Mod. No. 2-A-8	1 Each
604	24" x 20" Pipe Bend	1 Each

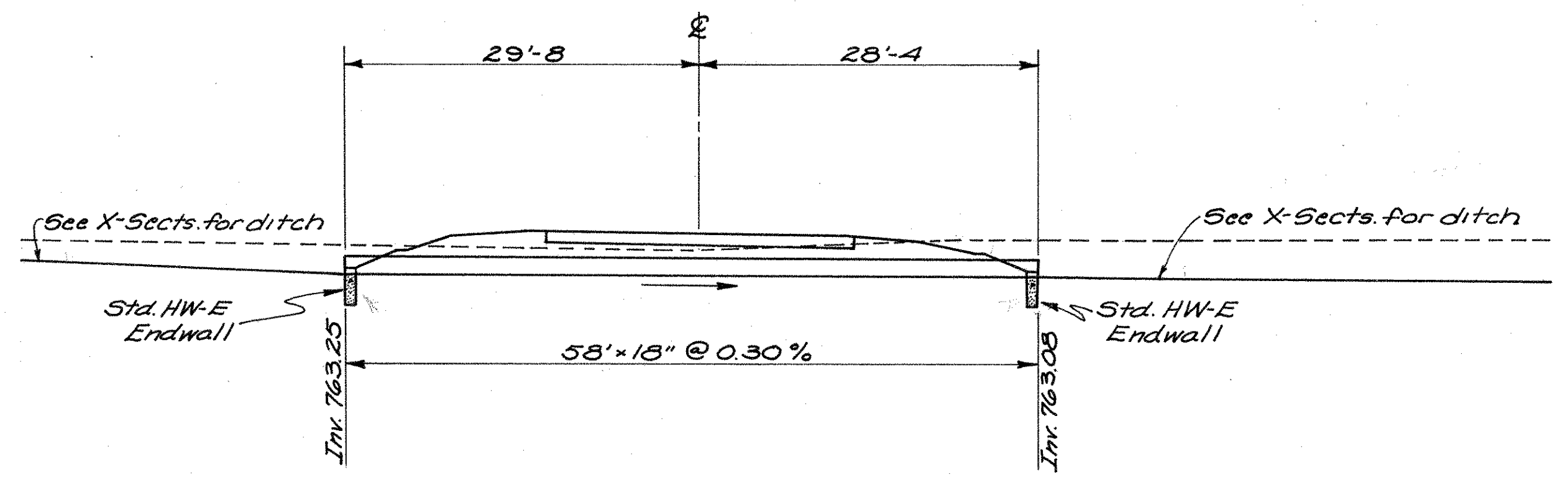


SECTION ON & PIPE



D.A. 2 Ac.
 Q25 T.c.f.s.

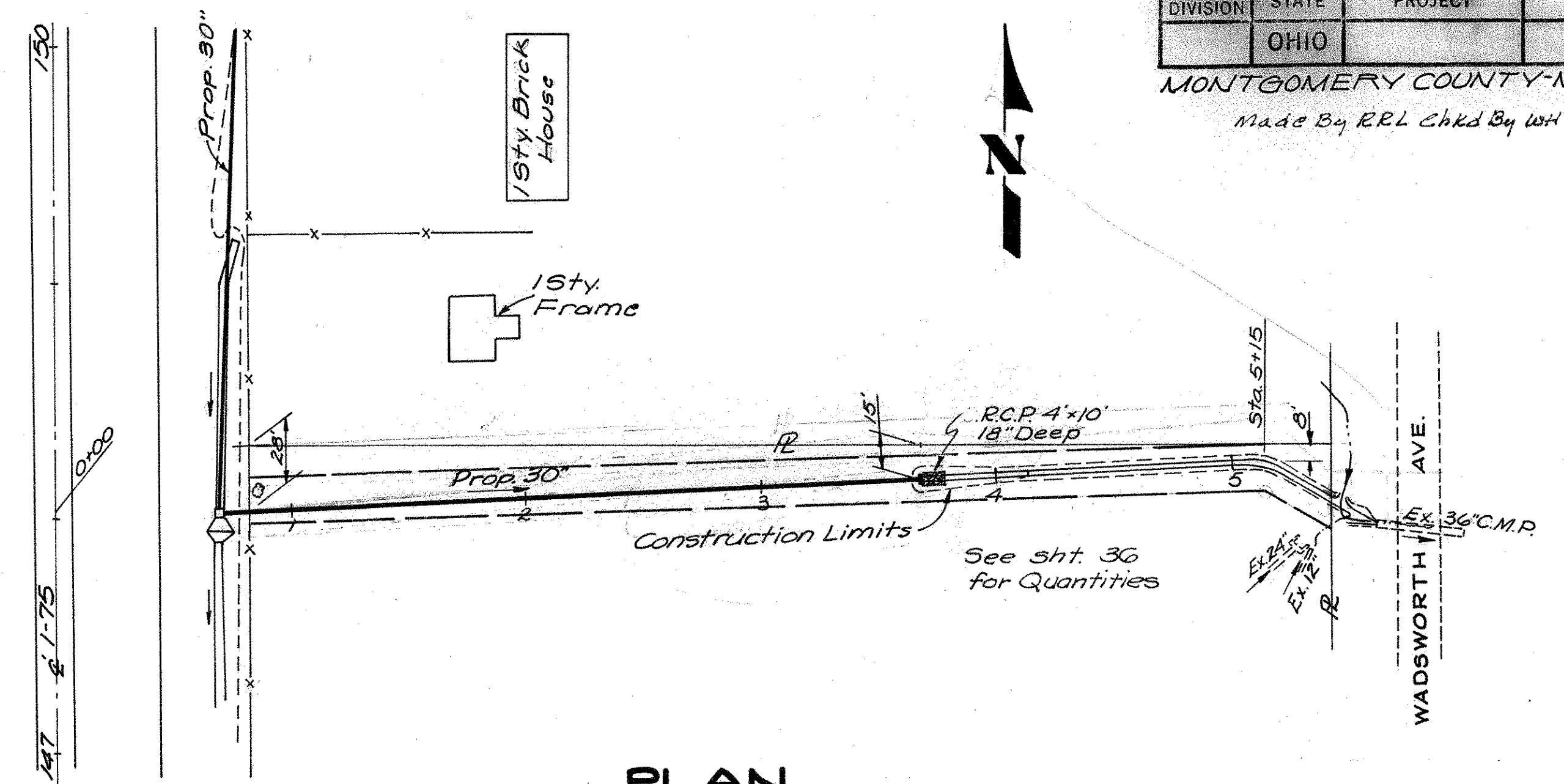
PLAN
 Scale: 1"=10'



SECTION ON PIPE
 Scale: 1"=10'

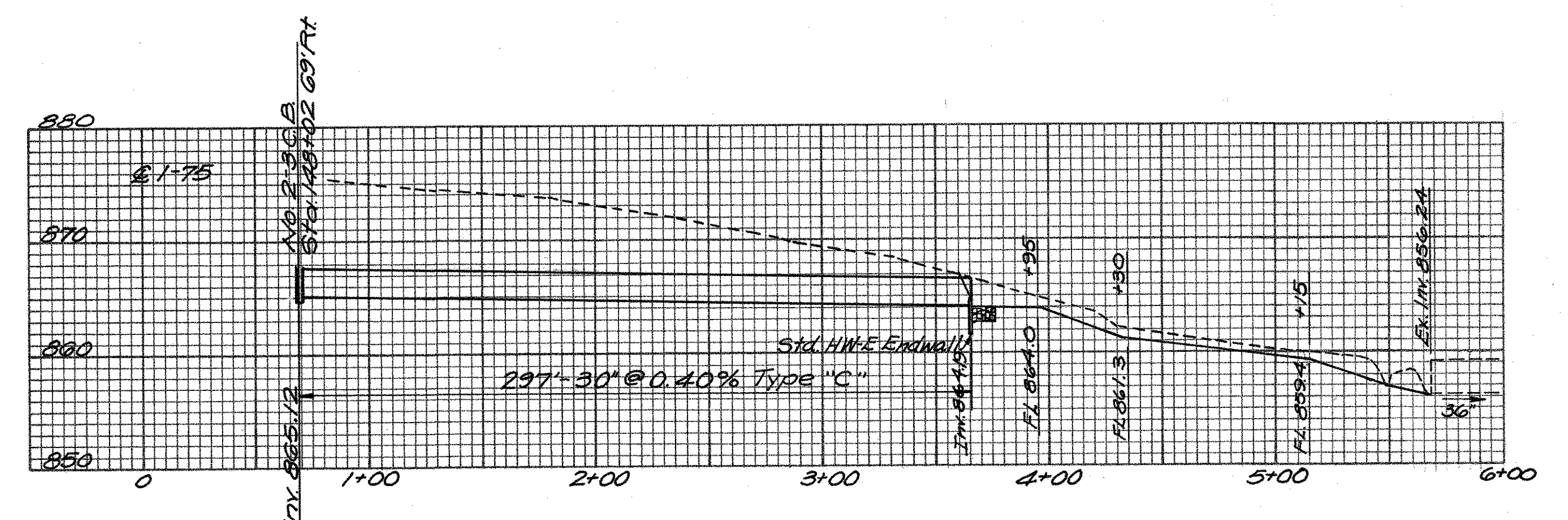
ESTIMATED QUANTITIES

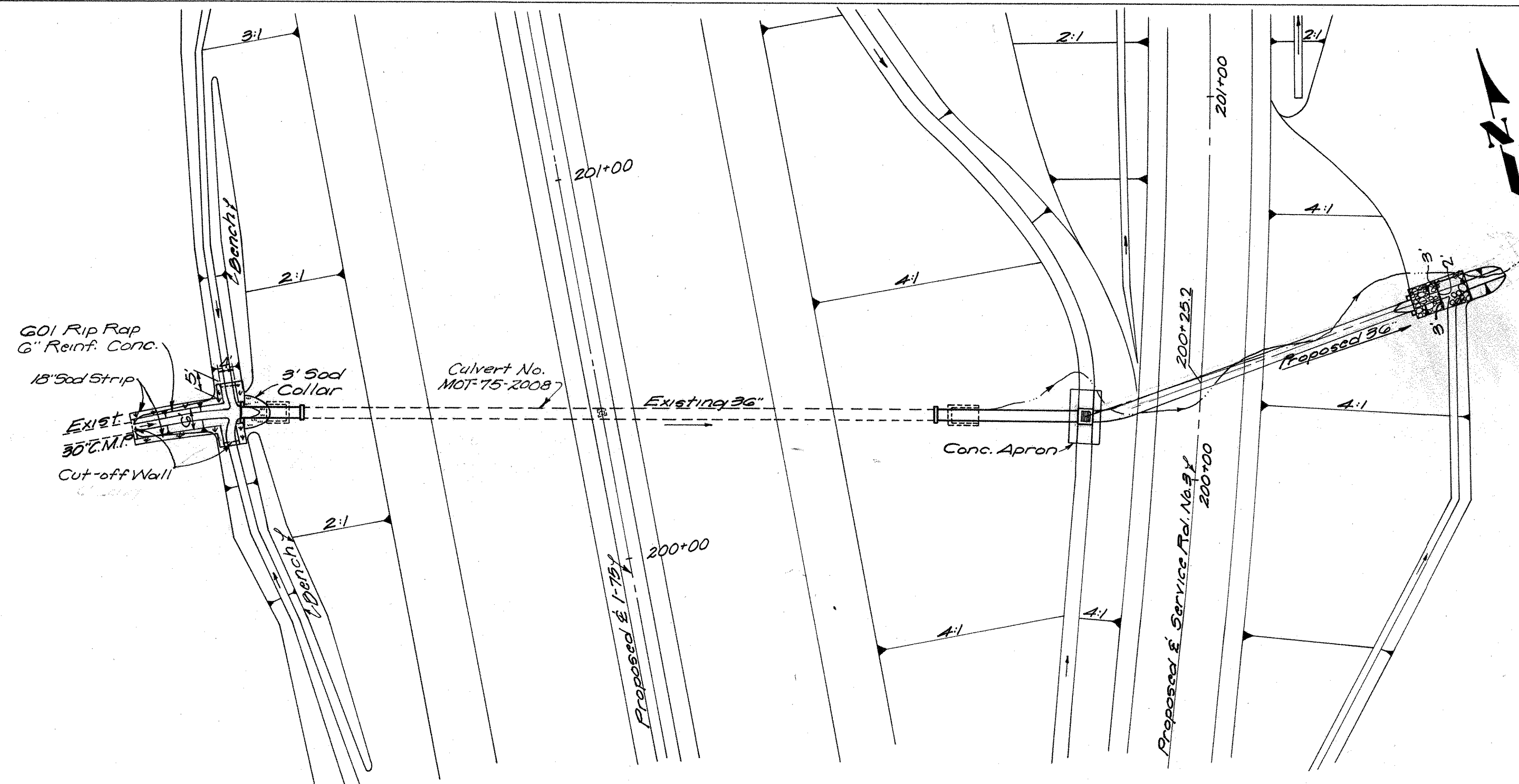
602 Concrete Masonry	0.60 C.Y.
603 18" Conduit Type "A" 706.01, 706.02 or 706.08	58 Lin. Ft.
660 Sodding	4 S.Y.



PLAN
STORM SEWER OUTLET
STA. 148+02 Rt.
 Scale 1"=50'

$$\frac{2 \times 4 \times 10}{27} = \frac{80}{27} = 2.96$$



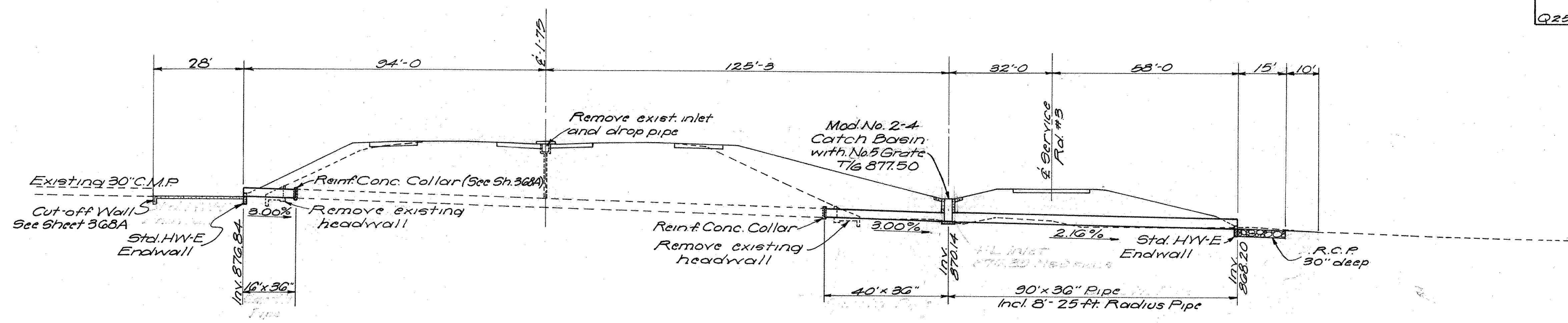


PLAN
 Scale 1" = 20'

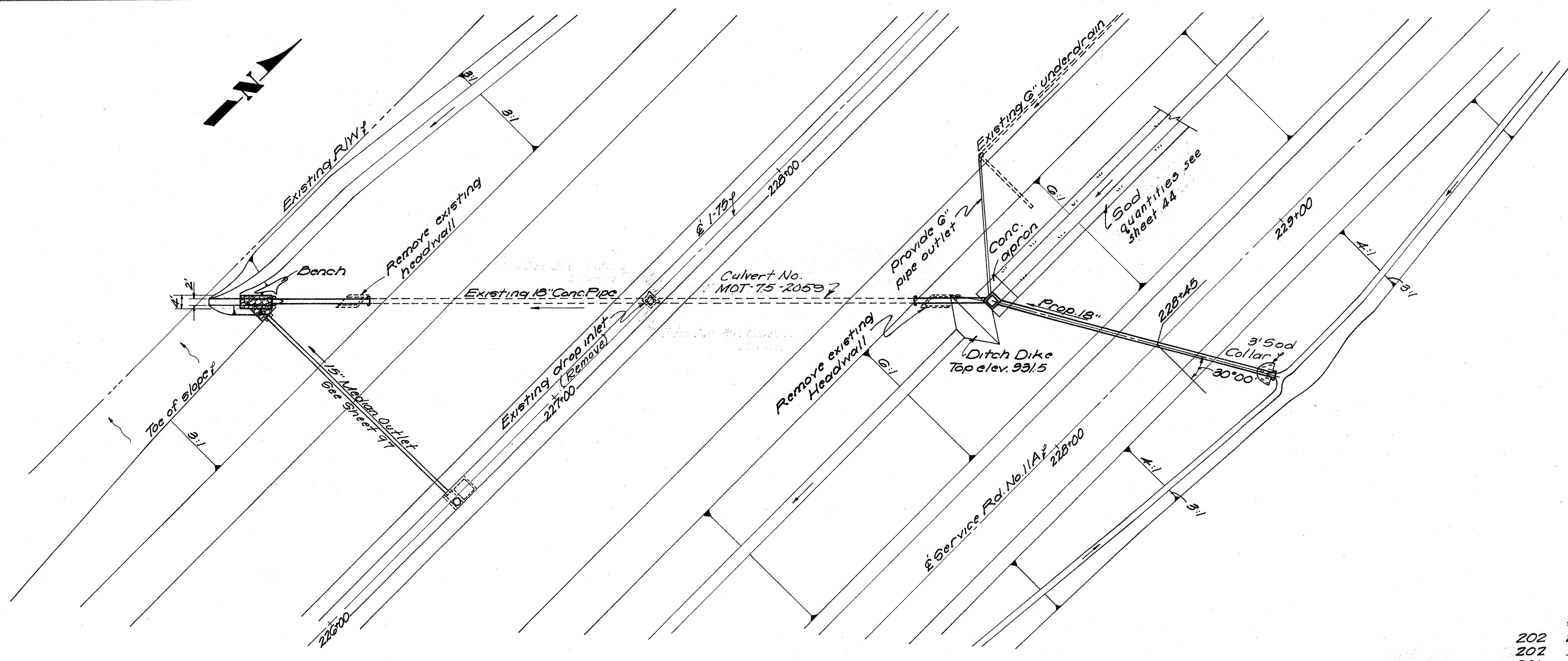
ESTIMATED QUANTITIES

202	Pipe Removed over 24"	8	Lin. Ft.
203	Excavation	3	Cu. Yds.
601	Rip Rap Using 6" Reinf. Conc.	13	Sq. Yds.
601	Type B Rock Channel Protection	14	Cu. Yds.
602	Concrete Masonry	12	Cu. Yds.
603	36" Conduit, Type "A" 706.02 or 706.08	138	Lin. Ft.
603	36" Conduit, Type "A" 706.02, 25' Radius	8	Lin. Ft.
604	Catch Basin, 2-4 Modified as per plan	1	Each
600	Sodding	11	Sq. Yds.
202	Inlets Removed	1	Each

DA	32 Ac.
Q25	63 cfs.



SECTION ALONG PIPE

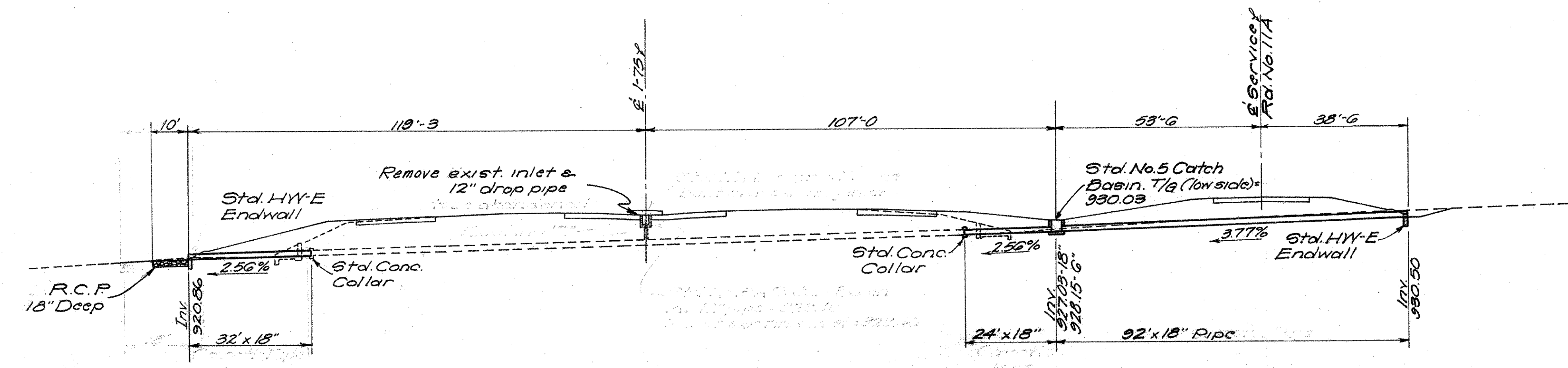


PLAN
Scale 1"=20'

ESTIMATED QUANTITIES

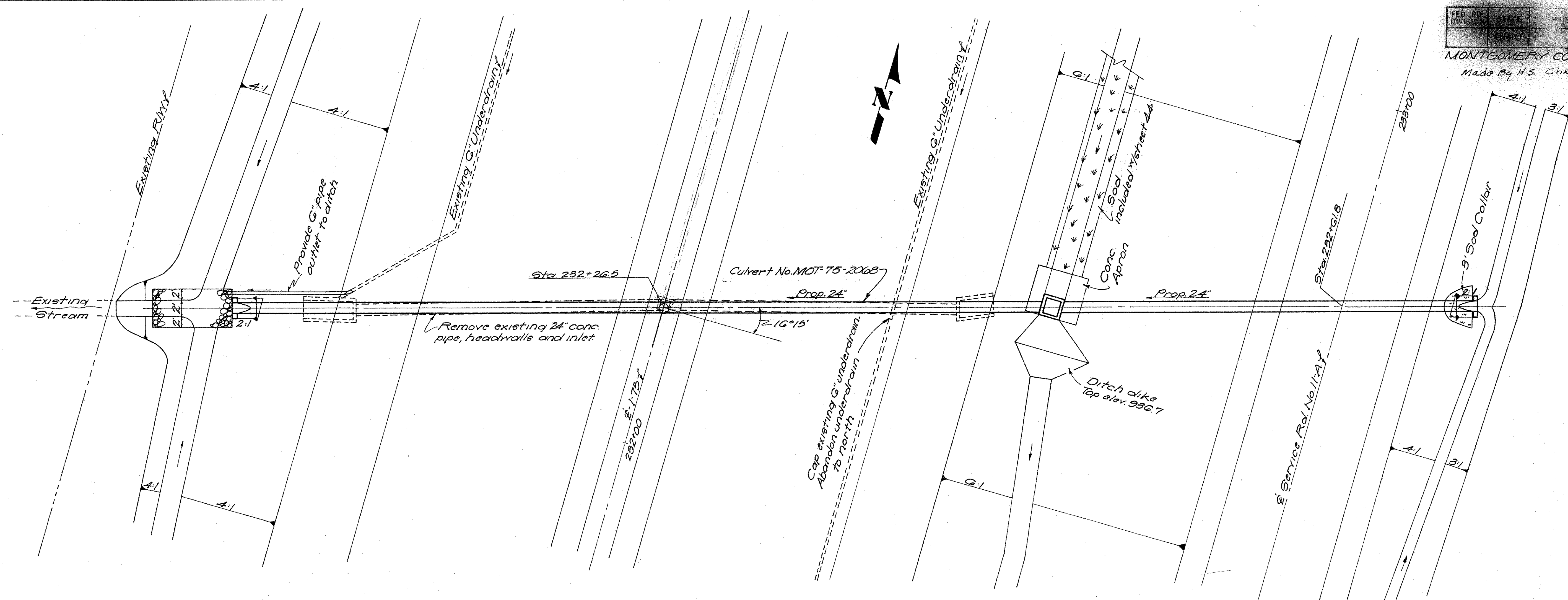
202	Pipe Removed 24" and under	8 Lin. Ft.
202	Inlets Removed	1 Each
601	Type B Rock Channel Protection	3.5 Cu. Yds.
602	Concrete Masonry	0.6 Cu. Yds.
603	18" Conduit, Type "A" 706.01, 706.02 or 706.08	148 Lin. Ft.
604	Catch Basins, Std. No. 5	1 Each
660	Sodding	399 Yds.

DA	6 Ac.
Q25	18 c.f.s.



SECTION ALONG PIPE

32
24
92
148



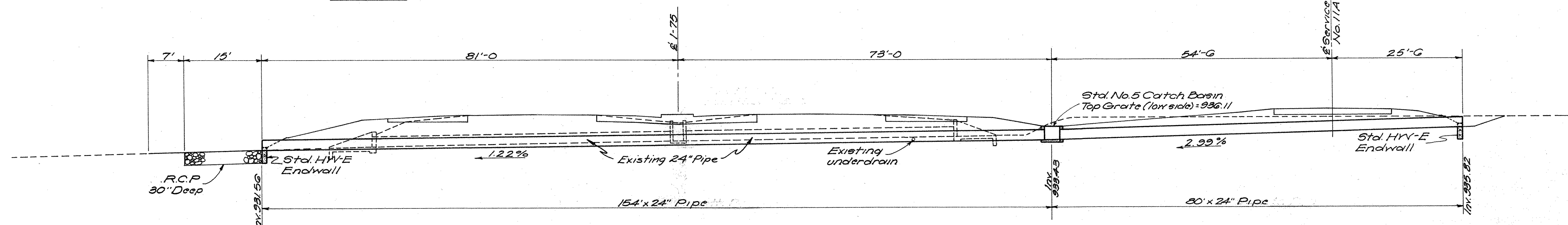
ESTIMATED QUANTITIES

202	Pipe Removed 24" and under	114 Lin. Ft.
203	Excavation	2 Cu. Yds.
G01	Type B Rock Channel Protection	10 Cu. Yds.
G02	Concrete Masonry	0.8 Cu. Yds.
G03	24" Conduit, Type "A" 706.02 or 706.08	234 Lin. Ft.
G04	Catch Basins, Std. No. 5	1 Each
G00	Sodding	3 Sq. Yds.
202	Inlets Removed	1 Each

PLAN
 Scale 1" = 10'

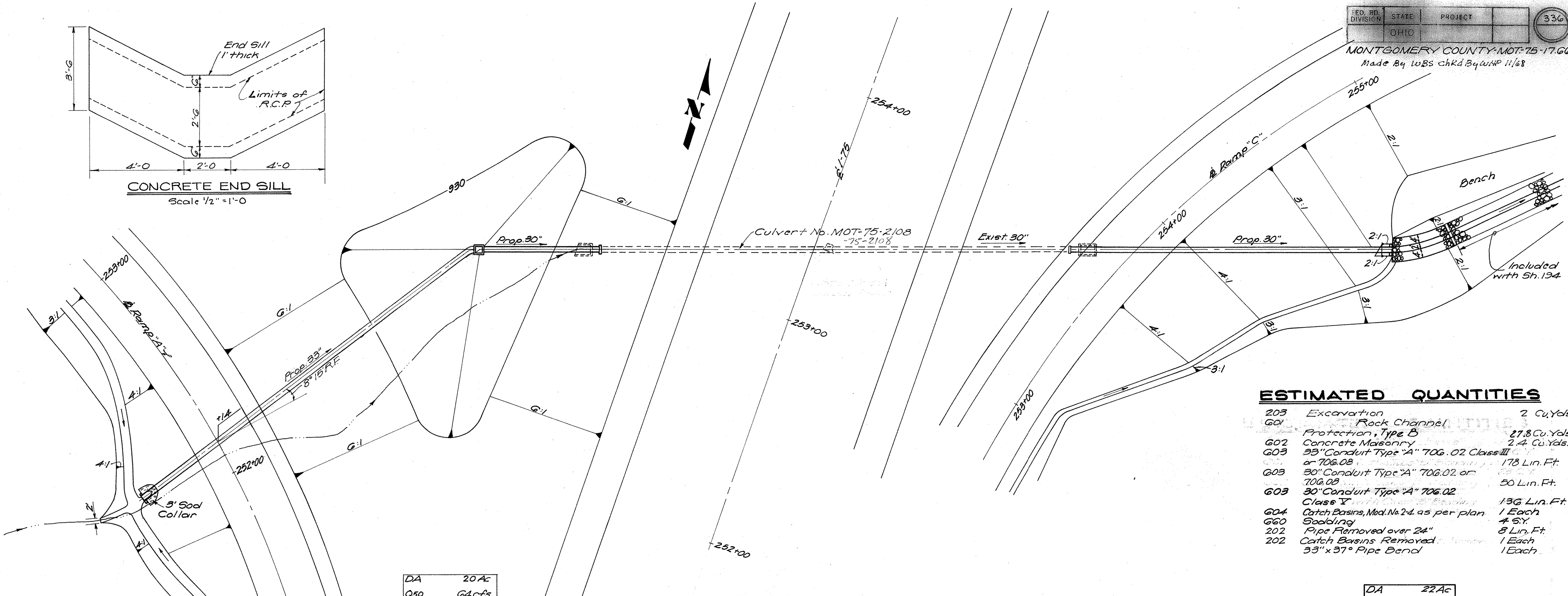
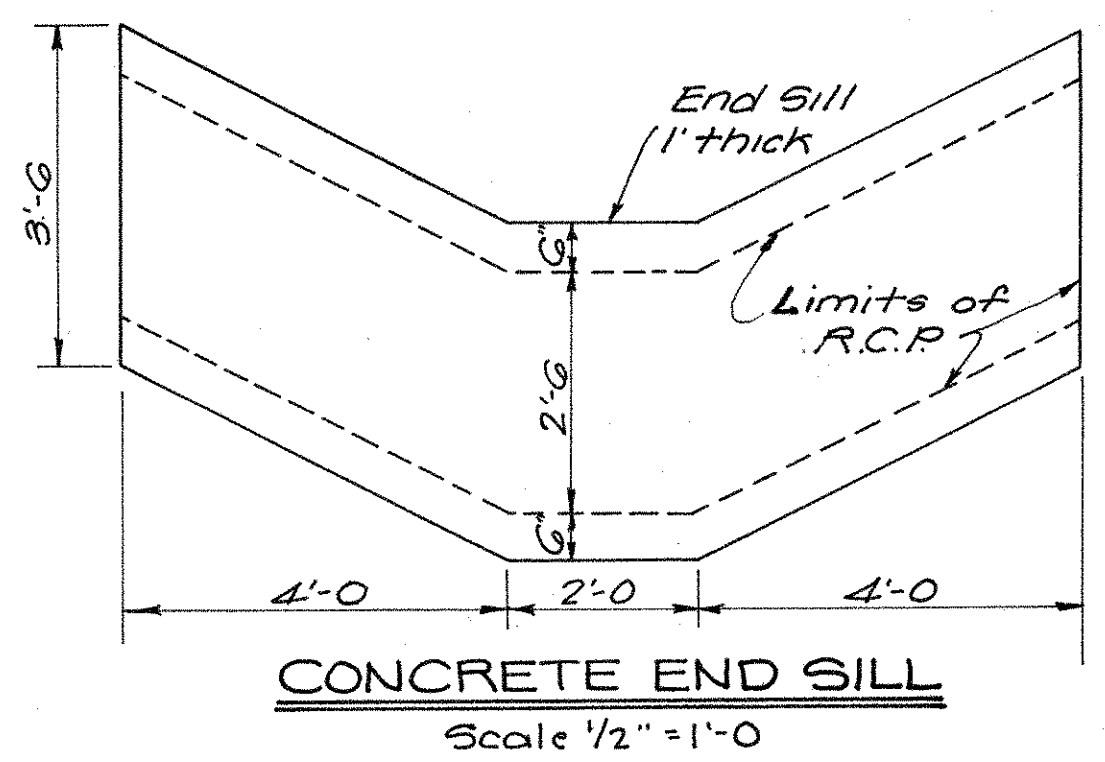
DA	7 Ac.
Q25	18 cfs

DA	9 Ac.
Q50	25 cfs



SECTION ALONG PIPE

PIPE CULVERT STA. 232+26.5 I-75 & STA. 232+61.8 SERVICE RD. NO. 11-A

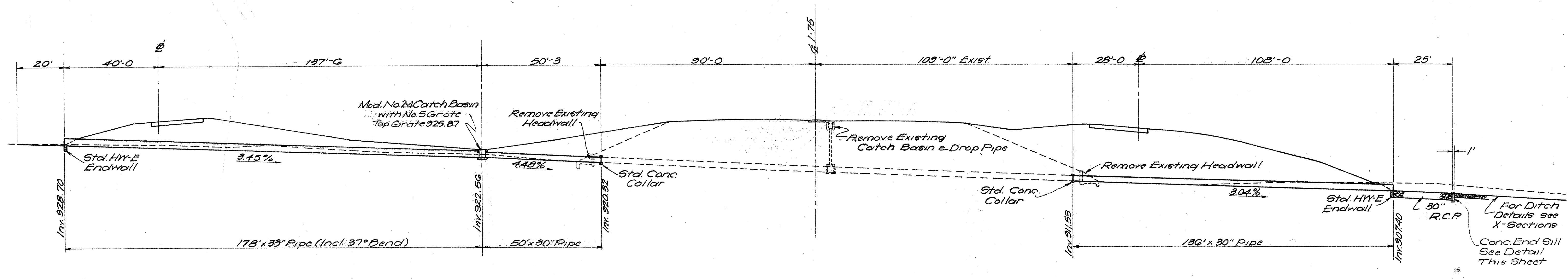


DA 20 Ac
 Q50 64 cfs

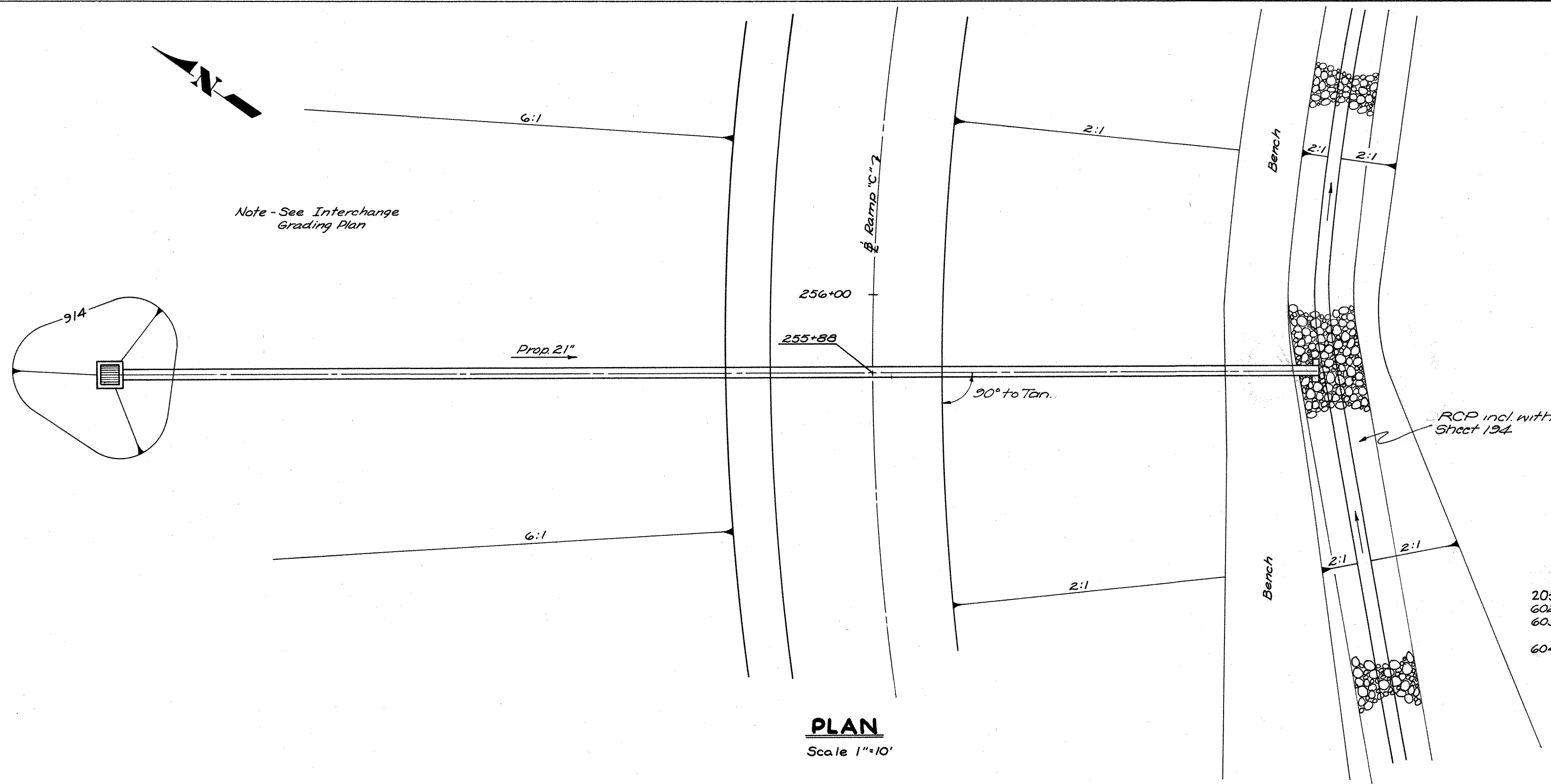
DA 22 Ac
 Q50 72 cfs

ESTIMATED QUANTITIES

203	Excavation	2 Cu. Yds.
601	Rock Channel Protection, Type B	27.8 Cu. Yds.
602	Concrete Masonry	2.4 Cu. Yds.
603	33" Conduit Type "A" 706.02 Class III or 706.08	178 Lin. Ft.
603	30" Conduit Type "A" 706.02 or 706.08	50 Lin. Ft.
603	30" Conduit Type "A" 706.02 Class V	136 Lin. Ft.
604	Catch Basins, Mod. No. 24 as per plan	1 Each
660	Sodding	4 S.Y.
202	Pipe Removed over 24"	8 Lin. Ft.
202	Catch Basins Removed	1 Each
	33" x 37° Pipe Bend	1 Each



SECTION ON E PIPE

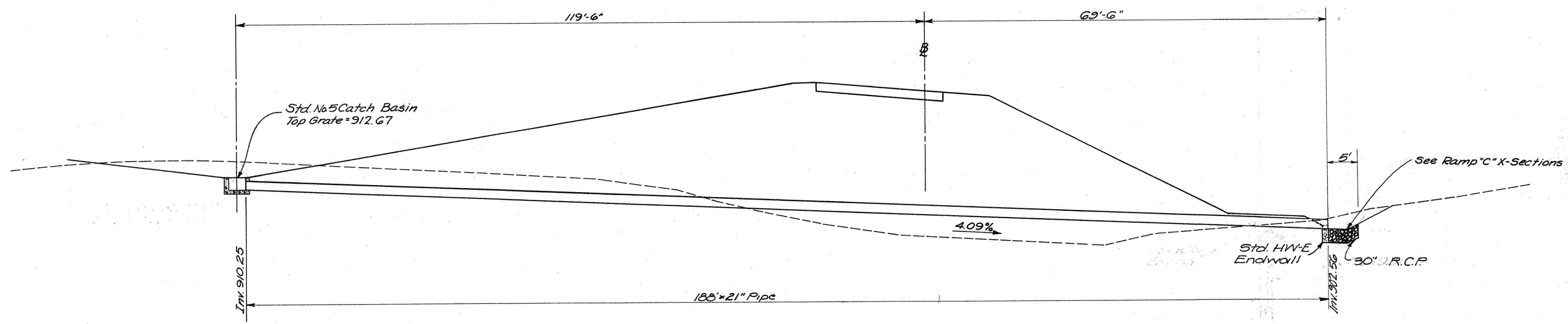


PLAN
 Scale 1"=10'

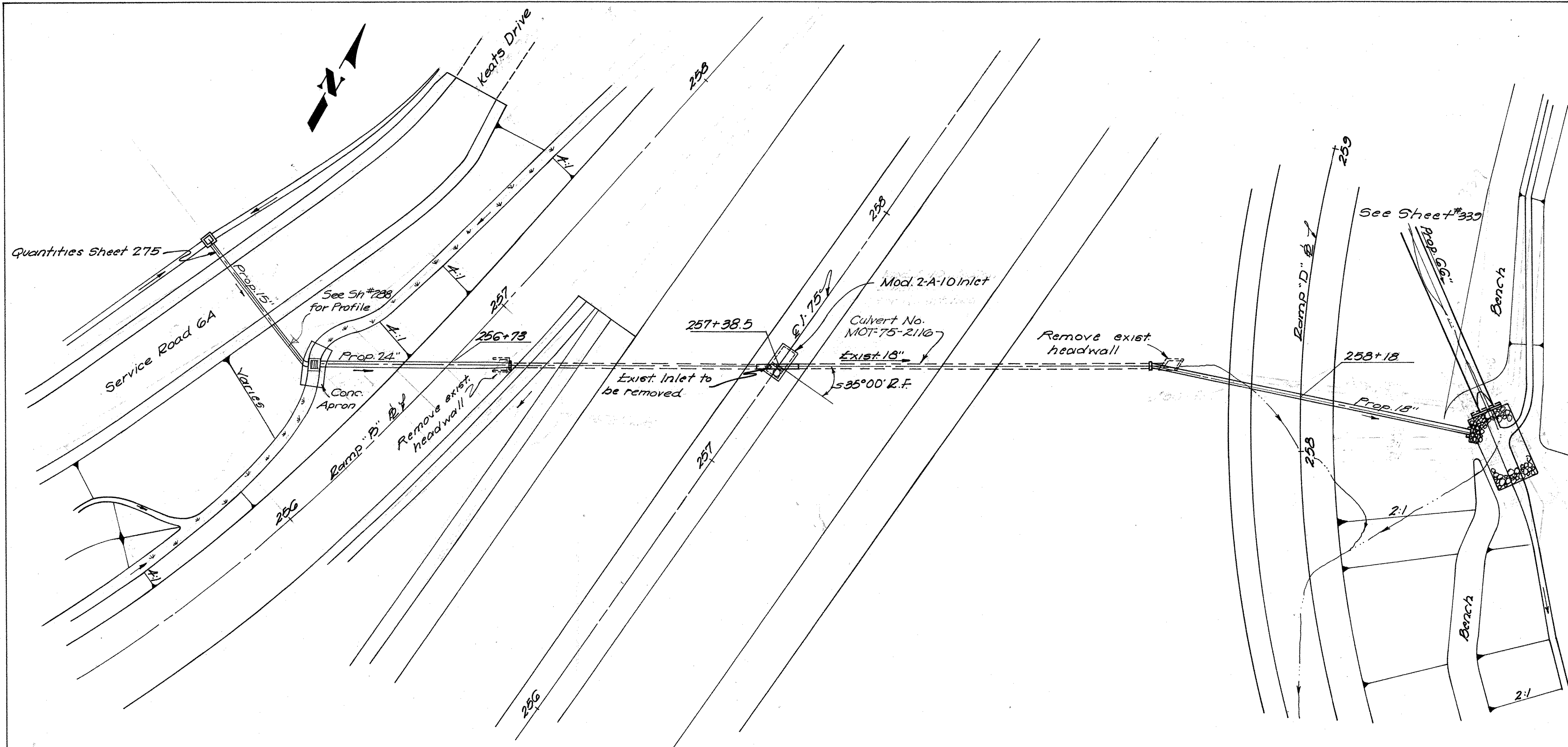
ESTIMATED QUANTITIES

205	Excavation	2 C.Y.
602	Concrete Masonry	04 C.Y.
603	21" Conduit Type "A" 706.02 Class II	188 L.F.
604	Catch Basin, Std. No. 5	1 Each

DA	30 Ac.
Q50	15 c.f.s.



SECTION ALONG C PIPE

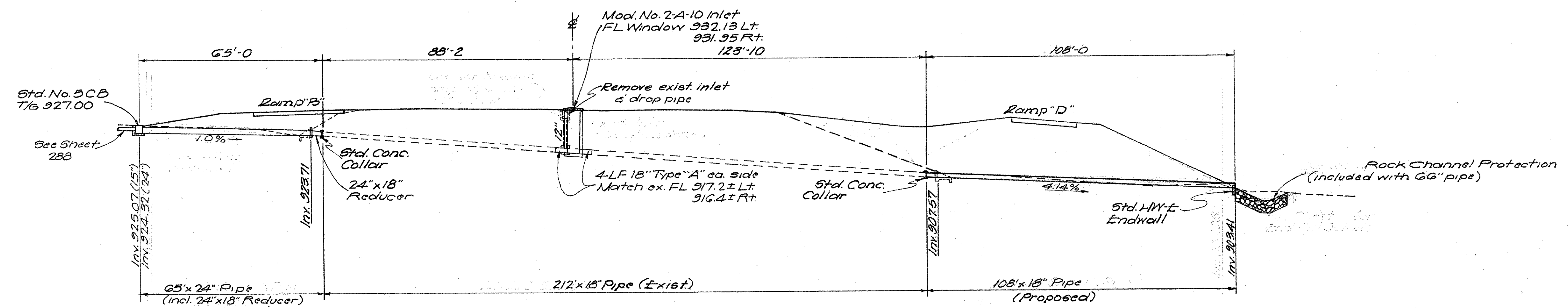


ESTIMATED QUANTITIES

202-Existing Structures Removed	Lump Sum
G02-Concrete Masonry	0.6 C.Y.
G03-24" Conduit Type "A" 706.02 or 706.08	65 L.F.
18" Conduit Type "A" 706.02 Class X	116 L.F.
G60-Sodding	4.5 Y.
202-Pipe Removed 24" and under	8 Lin. Ft.
202-Inlets Removed	1 Each
G04-Mod. 2-A-10 Inlet as per plan	1 Each

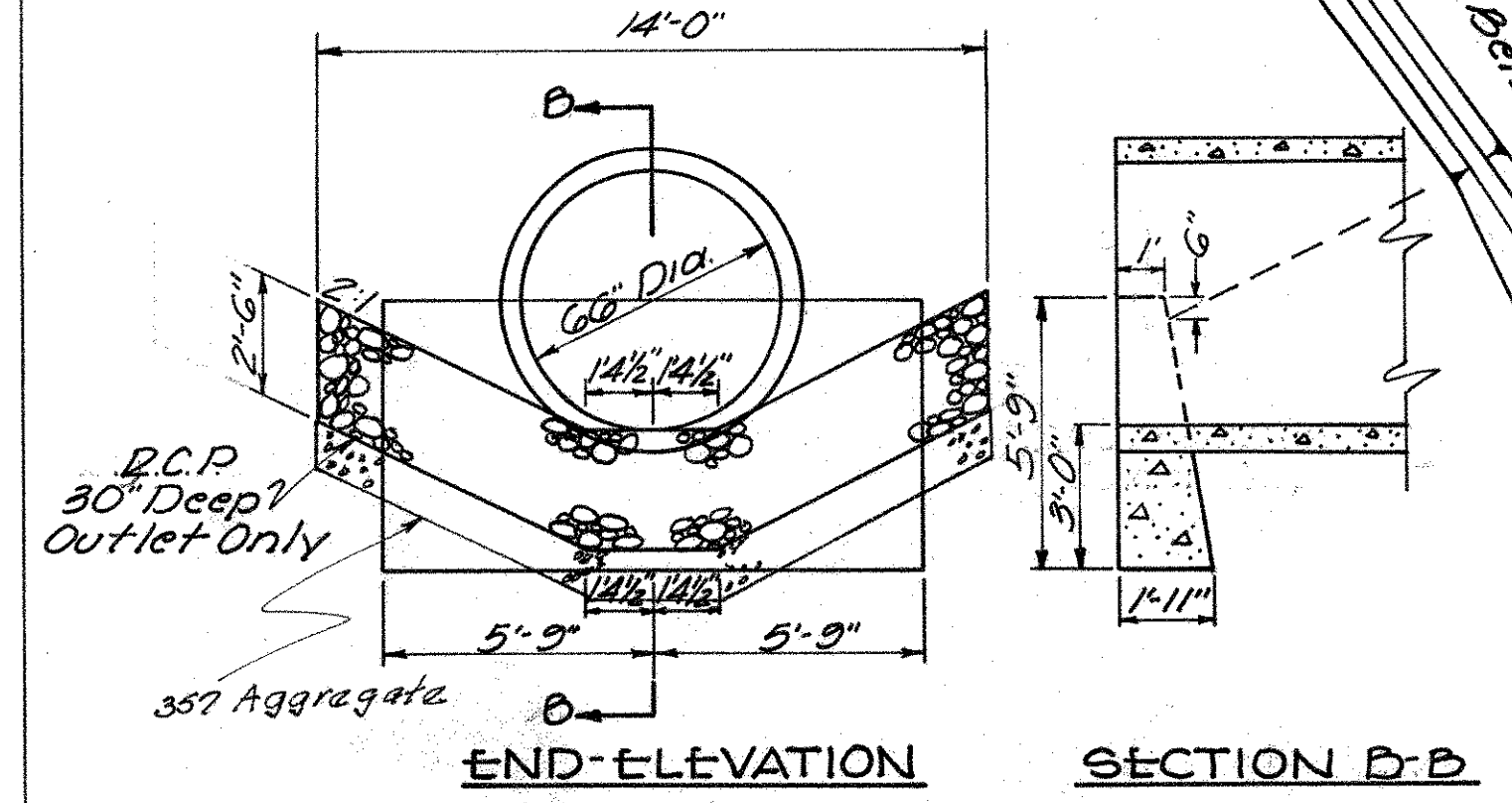
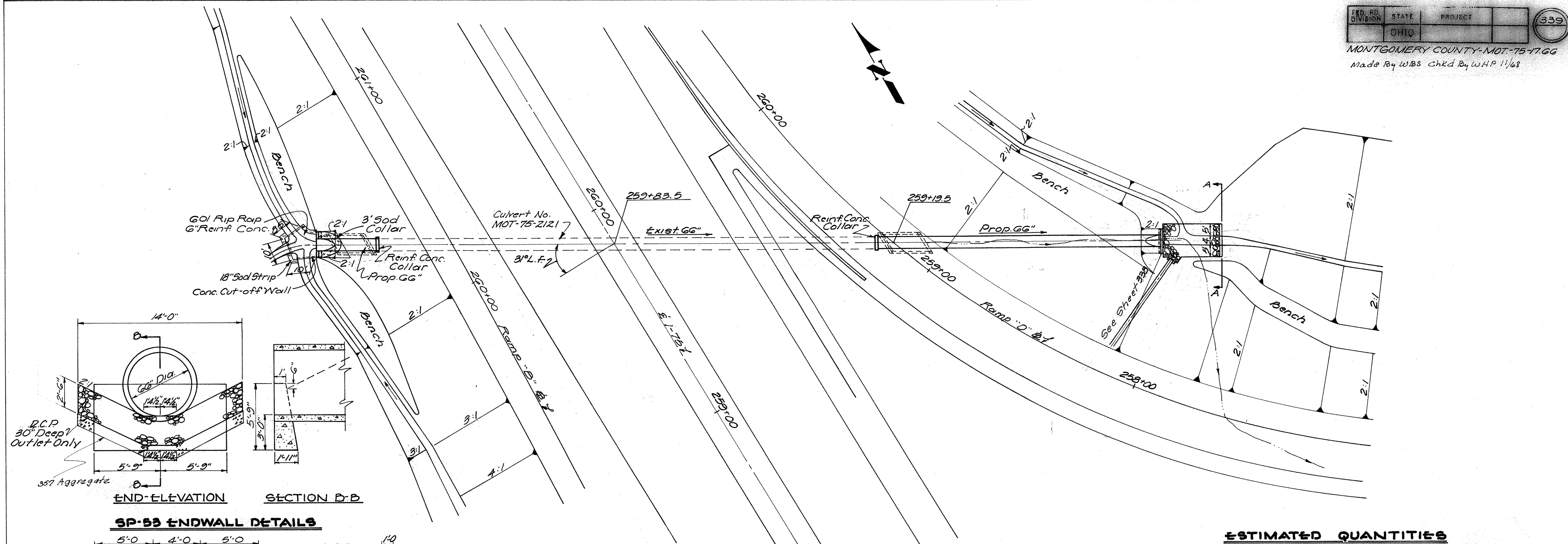
D.A.	4 Ac.
Q25	18 cfs

PLAN
Scale: 1" = 20'

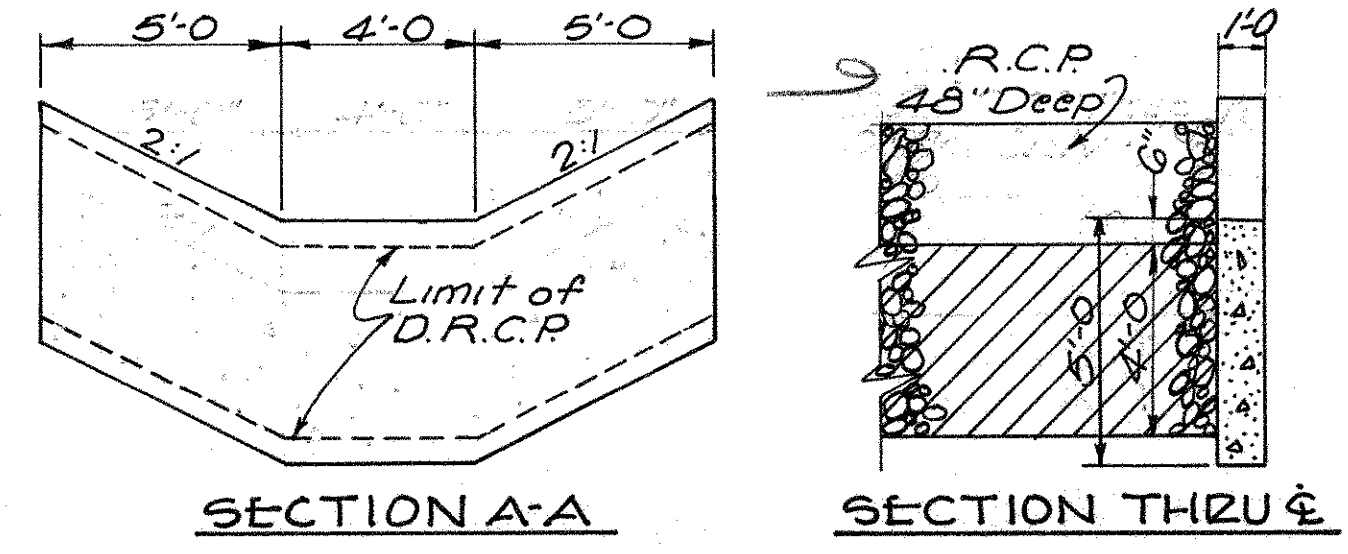


SECTION ON PIPE

PIPE CULVERT STA. 256+73 RAMP "B"
PIPE CULVERT STA. 257+38.5 1-75
PIPE CULVERT STA. 258+18 RAMP "D"



SP-53 ENDWALL DETAILS



CONCRETE END SILL

Scale: 1/4" = 1'-0"

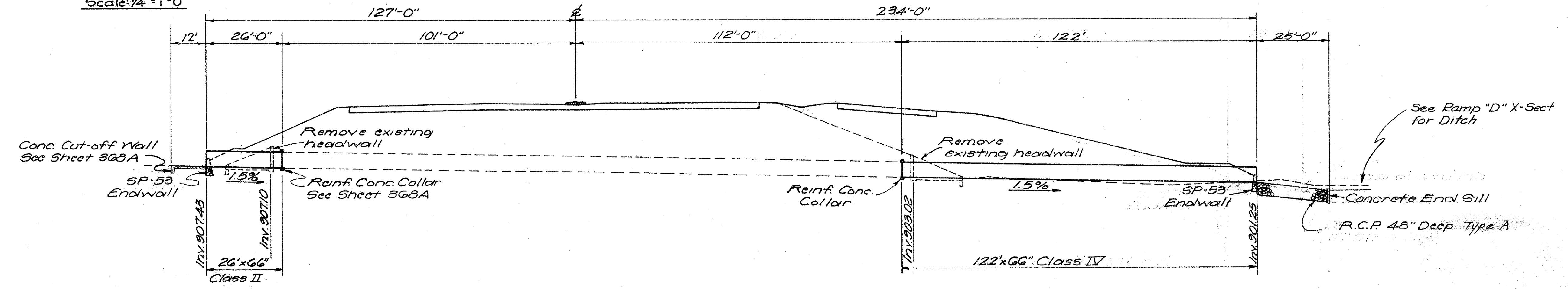
PLAN

Scale: 1" = 20'

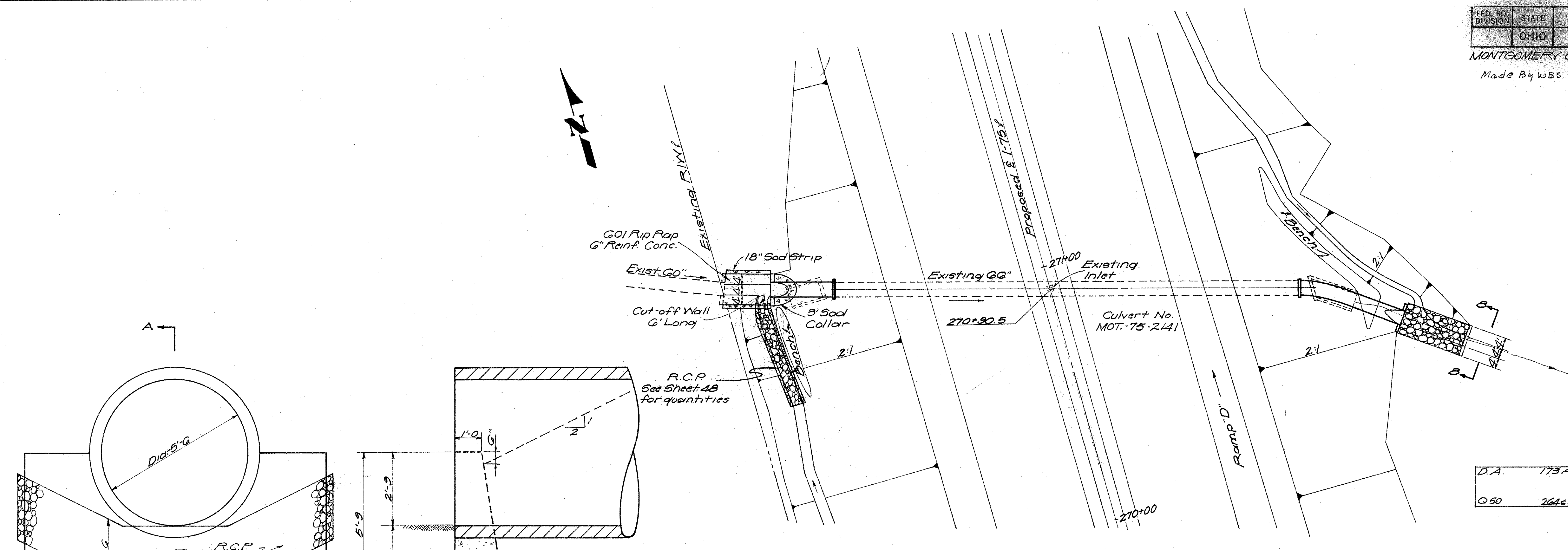
ESTIMATED QUANTITIES

202	Pipe Removed over 24"	8 Lin. Ft.
601	Rock Channel Protection Type A	583 C.Y.
601	Rip Rap using 6" Reinf. Conc.	16 S.Y.
602	Concrete Masonry	82 C.Y.
603	66" Conduit Type "A" 706.02 Class II	122 L.F.
	66" Conduit Type "A" 706.02	26 L.F.
660	Sodding	11 S.Y.

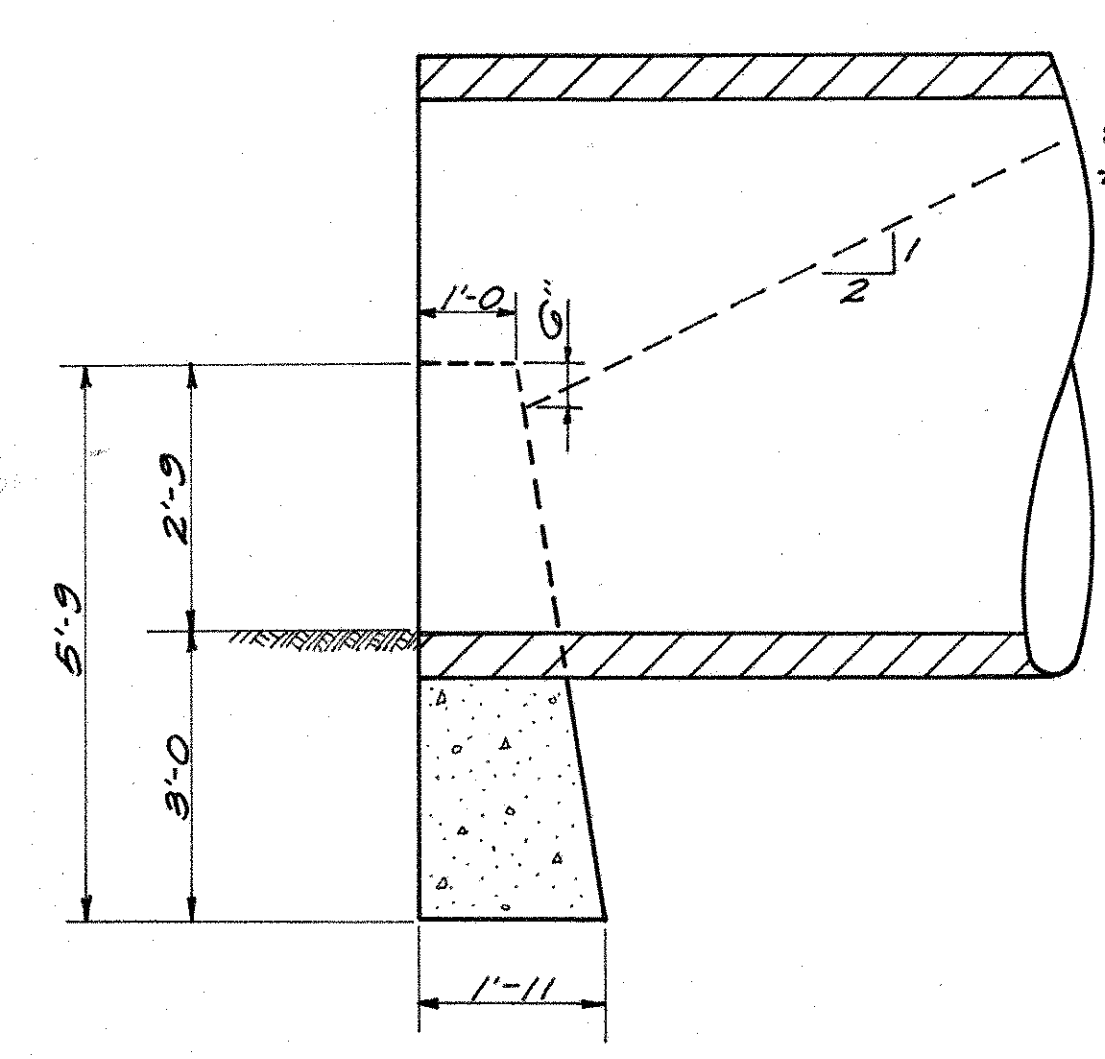
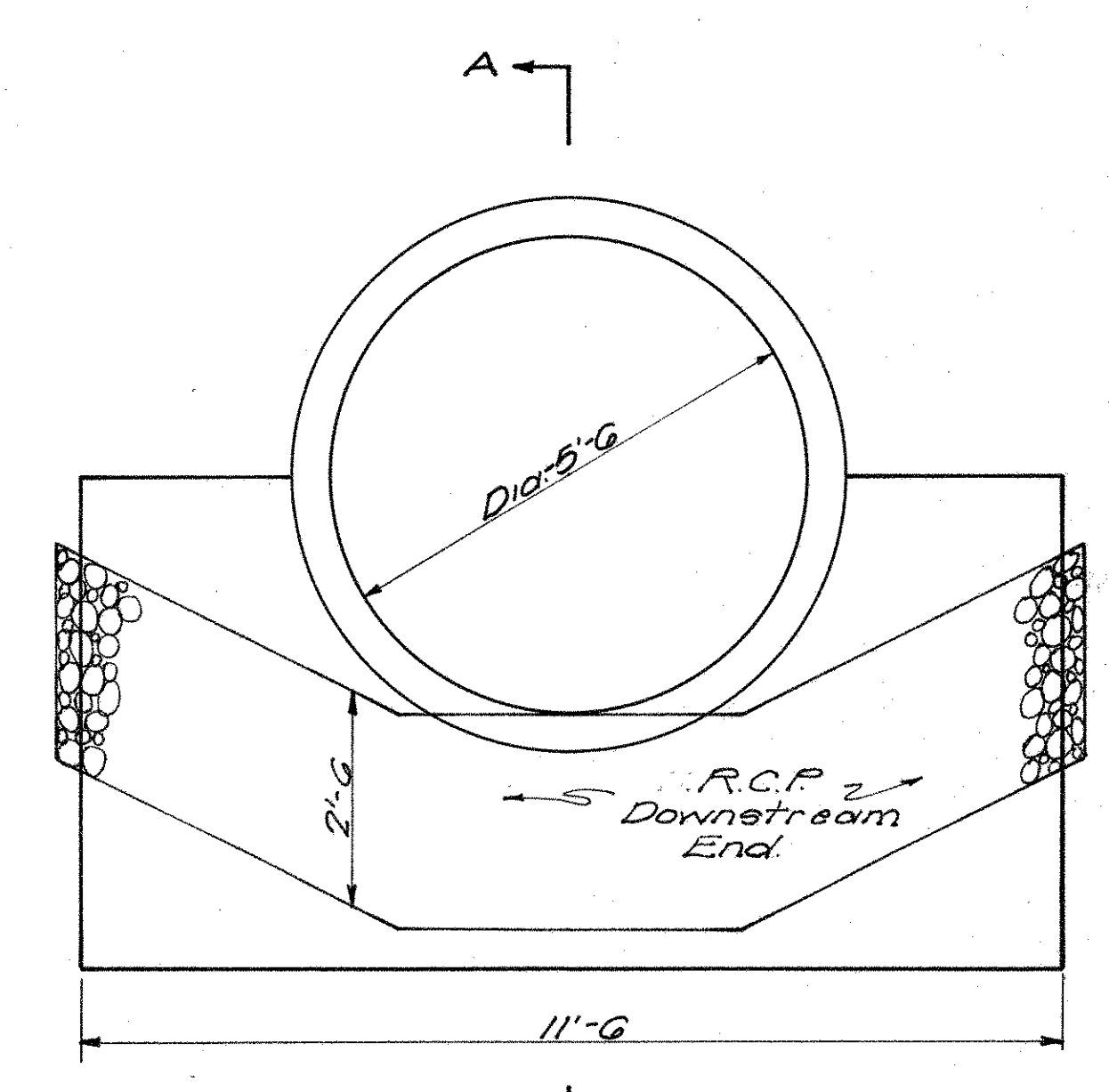
D.A.	137 Ac.
Q50	234 c.f.s.



SECTION ON & PIPE



D.A.	173 Ac.
Q 50	264 c.f.s.



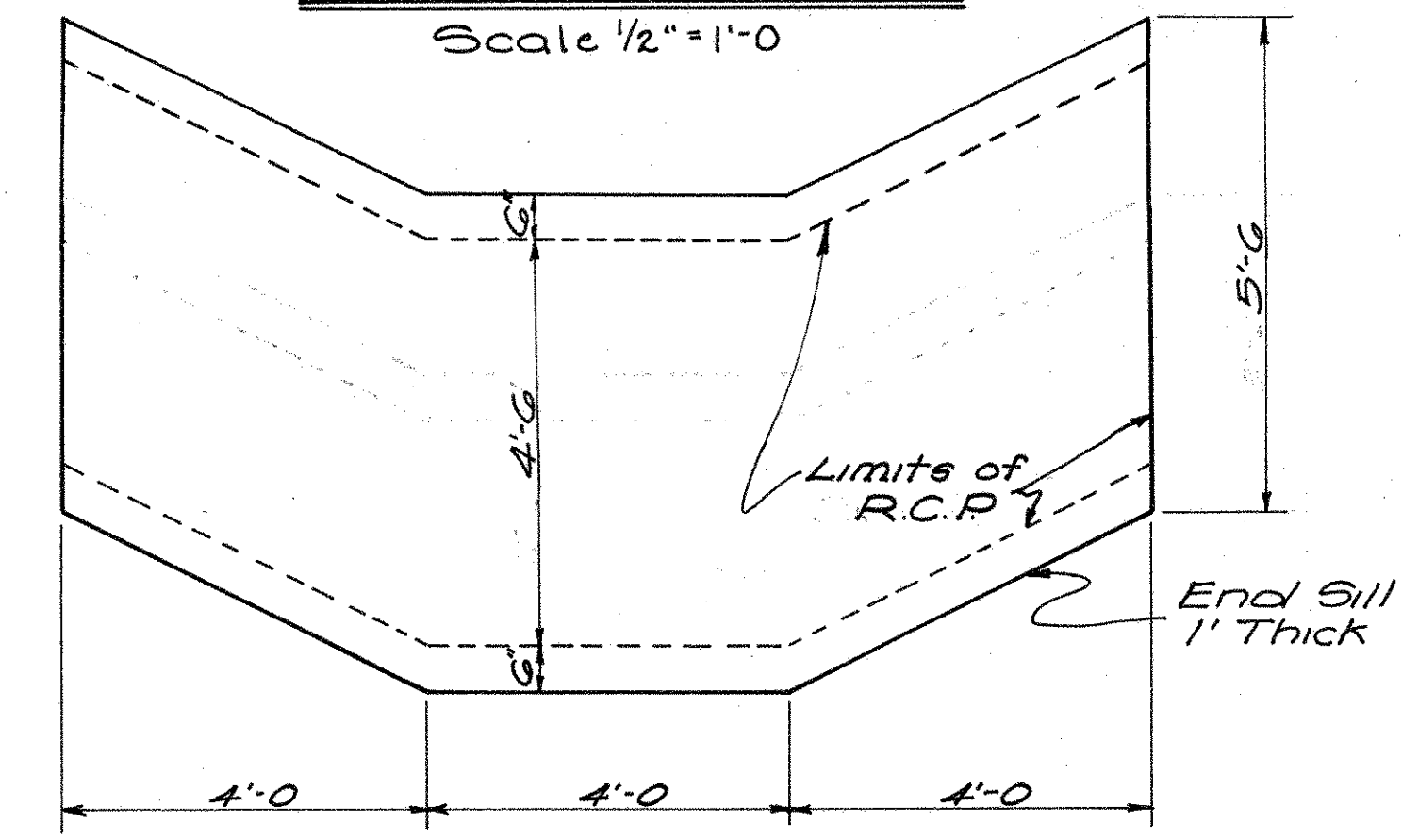
PLAN
Scale 1"=20'

ESTIMATED QUANTITIES

203	Excavation	1 Cu. Yd.
G01	Rock Channel Protection Type A	536 Cu. Yds.
G01	Rip Rap Using 6" Reinf. Concrete	24 Sq. Yds.
G02	Concrete Masonry	80 Cu. Yds.
G03	66" Conduit, Type 'A' 706.02, 50'	
	Radius with Class 'B' Bedding	20 Lin. Ft.
G03	66" Conduit, Type 'A' 706.02	
	with Class 'B' Bedding	46 Lin. Ft.
G60	Sodding	11 Sq. Yds.
202	Pipe Removed over 24"	8 Lin. Ft.
202	Inlets Removed	1 Each

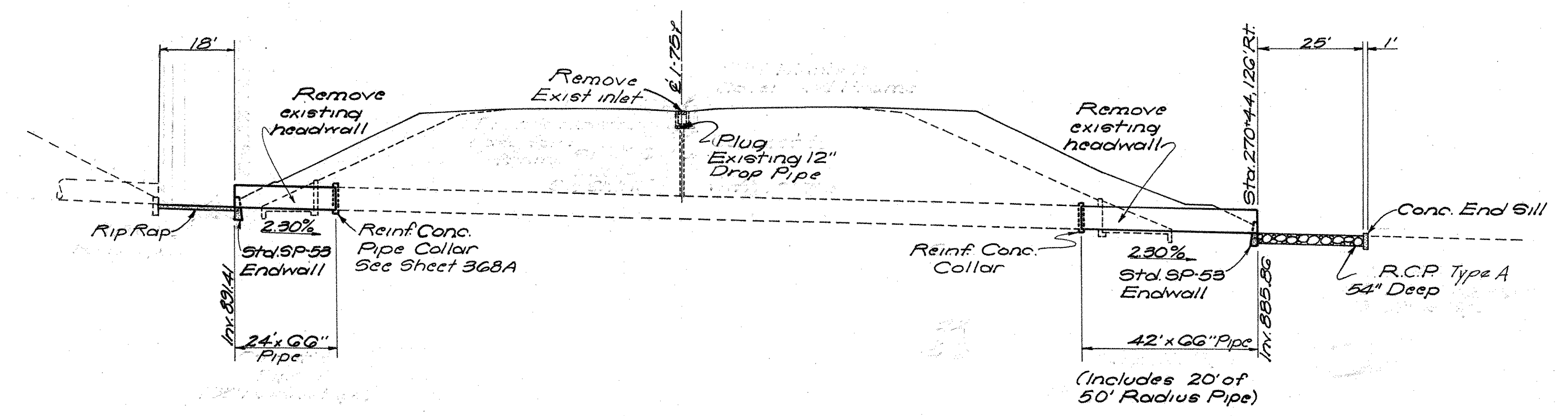
SP-53 END WALL

Scale 1/2"=1'-0"

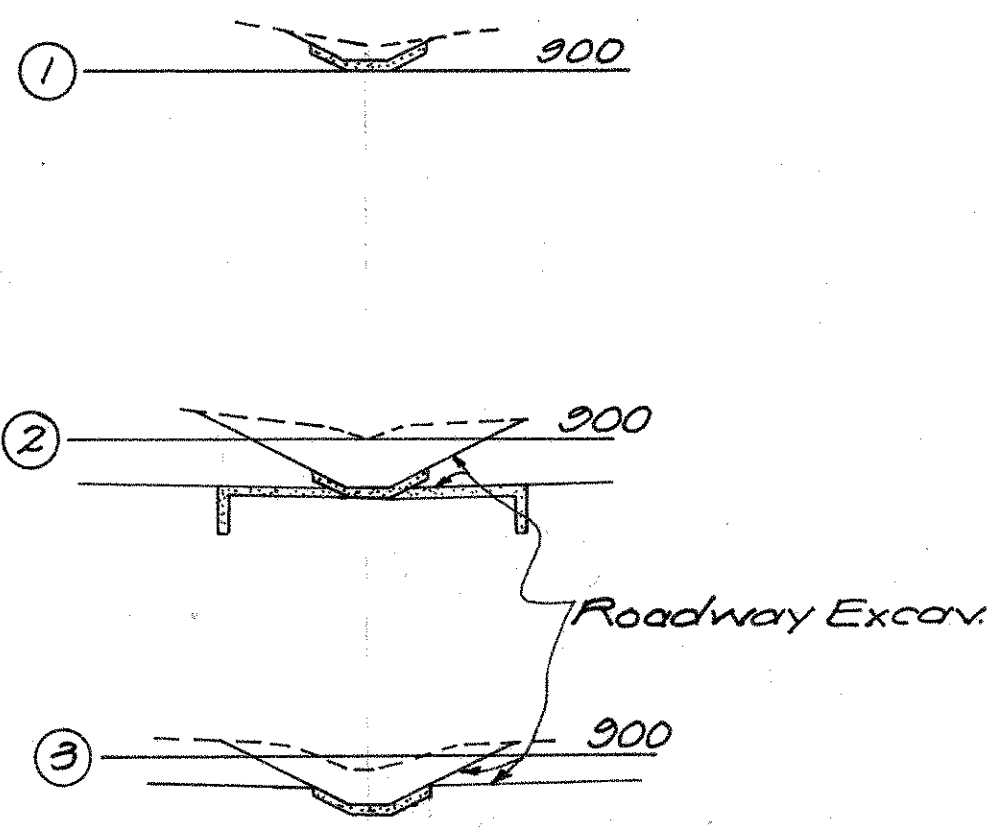
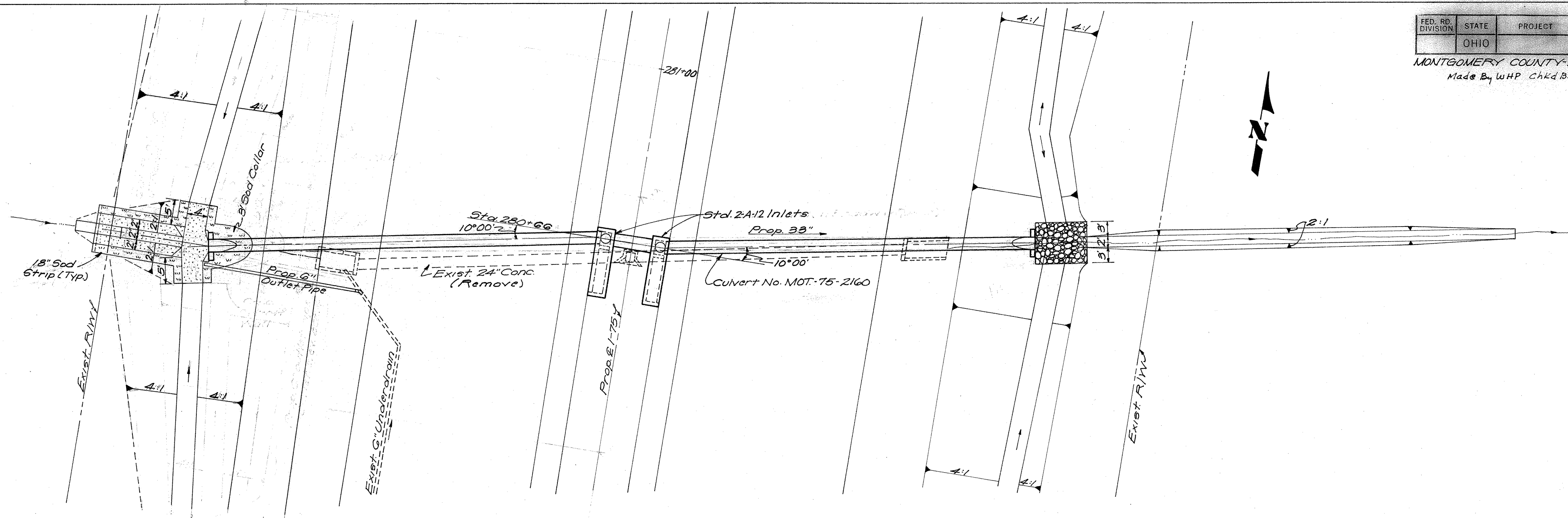


SECTION B-B
CONCRETE END SILL

SECTION A-A



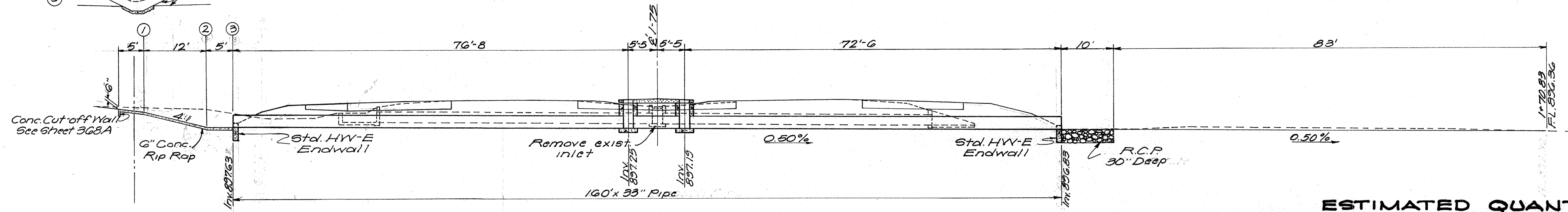
SECTION ALONG & PIPE



EXCAVATION			
Section	Area-ft ²	Dist.	Vol-ft ³
0	0	9	11
①	2.5	12	19.5
②	30	5	25
③	20		

PLAN
Scale 1"=10'

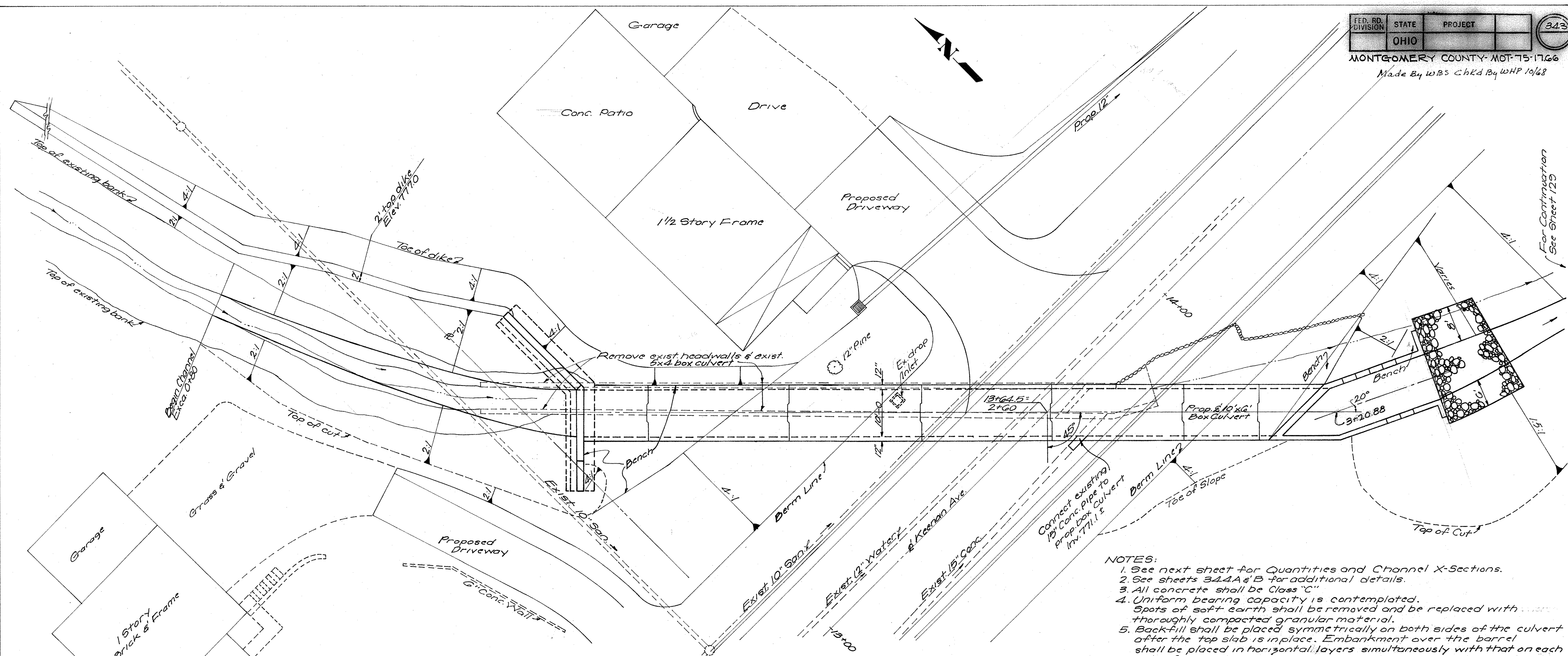
331 x 1/21 = 12.2 C.Y.



SECTION ALONG PIPE

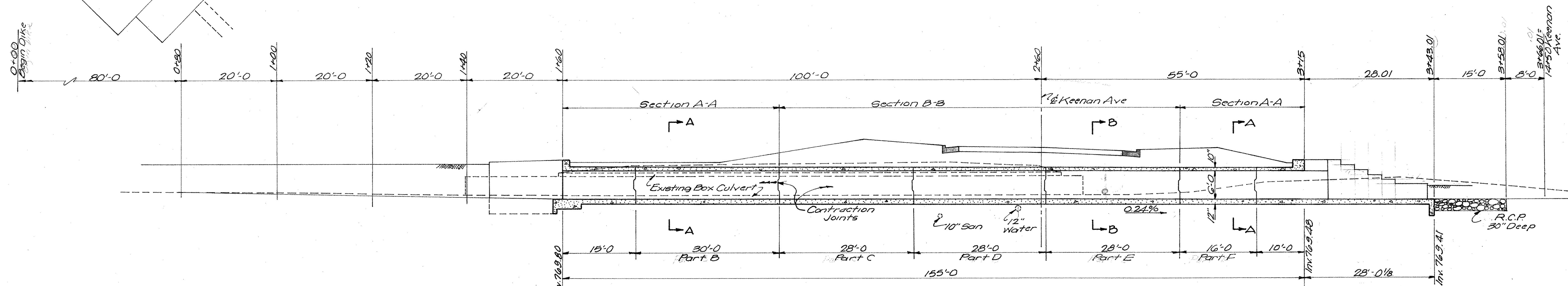
DA	85A.
Q50	38c.f.s.

ESTIMATED QUANTITIES		
202	Pipe Removed 24" and under	107 Lin. Ft.
202	Inlets Removed	1 Each
203	Excavation	15 Cu. Yds.
204	Std. 2-12 Inlets	2 Each
G01	Rip Rap using 6" Reinf. Conc.	19 Sq. Yds.
G01	Type B Rock Channel Protection	89 Cu. Yds.
G02	Concrete Masonry	11 Cu. Yds.
G03	33" Conduit, Type "A" 706.02 or 706.08 with Class "B" Bedding	160 Lin. Ft.
G60	Sodding	13 Sq. Yds.

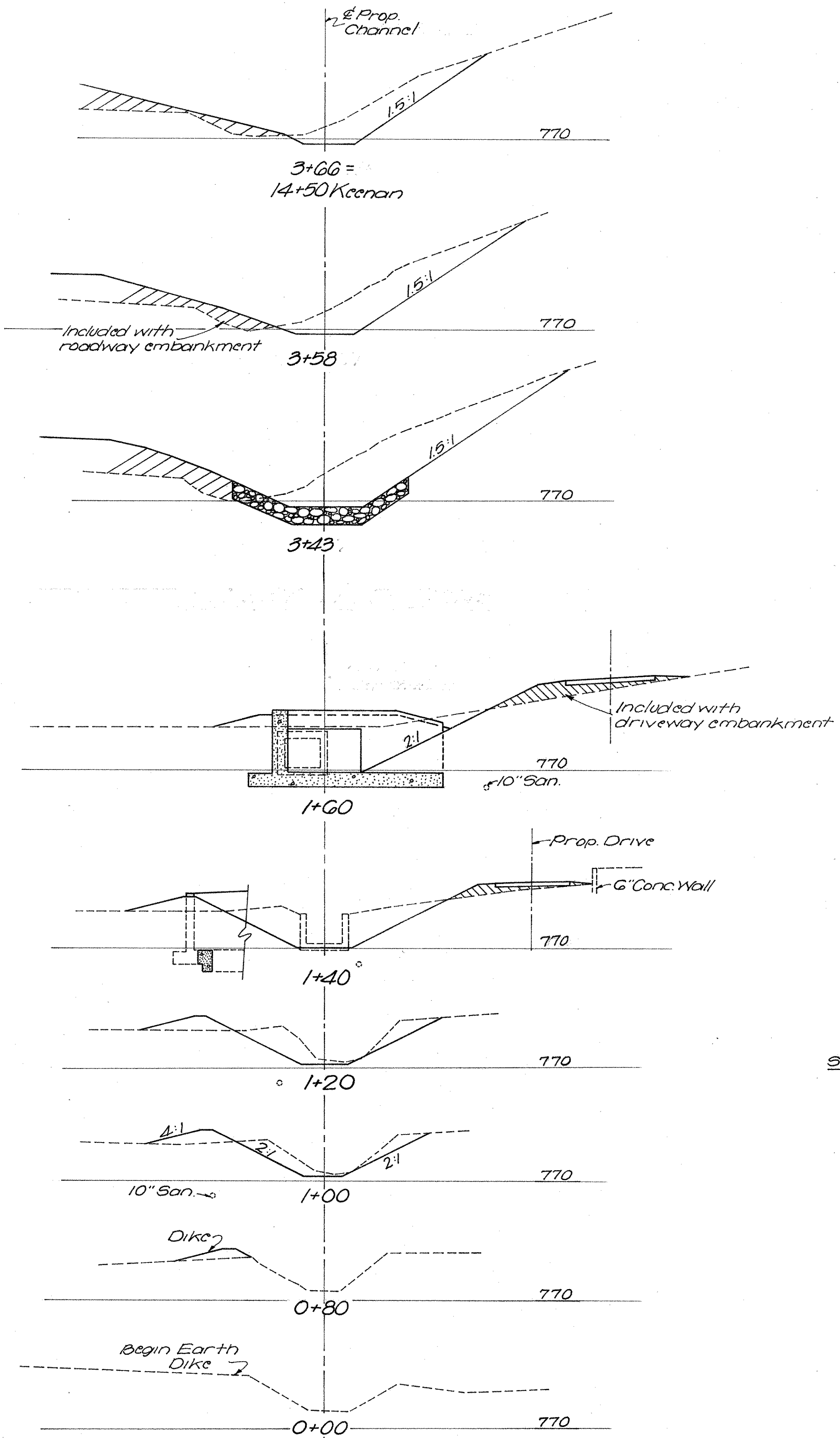


PLAN

- NOTES:**
1. See next sheet for Quantities and Channel X-Sections.
 2. See sheets 344A & B for additional details.
 3. All concrete shall be Class "C"
 4. Uniform bearing capacity is contemplated. Spots of soft earth shall be removed and be replaced with thoroughly compacted granular material.
 5. Back-fill shall be placed symmetrically on both sides of the culvert after the top slab is in place. Embankment over the barrel shall be placed in horizontal layers simultaneously with that on each side of culvert.



SECTION ALONG & BOX CULVERT



CHANNEL X-SECTIONS

REINFORCING STEEL SCHEDULE

MARK	NO.	LENGTH	TYPE	WEIGHT	MARK	NO.	LENGTH	TYPE	WEIGHT	MARK	NO.	LENGTH	TYPE	WEIGHT
B703	3	12'-0"	Bt.	74	B601	30	18'-2"	Bt.	819	T701	3	18'-2"	Bt.	111
B704	3	12'-1"	Bt.	74	B602	29	11'-6"	St.	501	B601	36	18'-2"	Bt.	382
B705	4	12'-3"	Bt.	100	T601	30	13'-10"	Bt.	623	B602	36	11'-6"	St.	622
B706	3	12'-4"	Bt.	76	T602	29	11'-6"	St.	501	B603	1	10'-6"	St.	16
B707	3	12'-6"	Bt.	77	B501	24	4'-0"	Bt.	100	B604	1	13'-6"	Bt.	20
B708	3	12'-7"	Bt.	77	T501	118	7'-4"	Bt.	903	B605	6	7'-3"	St.	65
B709	1	11'-10"	Bt.	24	T502	24	6'-3"	St.	156	B606	6	10'-3"	Bt.	92
B710	1	11'-6"	Bt.	24	T504	46	29'-8"	St.	1423	T602	10	11'-6"	St.	173
B711	1	11'-1"	Bt.	23	T506	8	5'-2"	Bt.	43	T604	10	13'-4"	Bt.	200
B601	14	18'-2"	Bt.	382	T507	8	3'-9"	Bt.	31	T605-20	16	Varies*	St.	160
B602	13	11'-6"	St.	225	T508	4	6'-2"	Bt.	26	T621-36	16	Varies*	St.	156
T601	14	13'-9"	Bt.	289	T509	4	5'-0"	Bt.	21	B501	40	4'-0"	Bt.	167
T602	14	11'-6"	St.	242	B401	29	16'-11"	Bt.	328	B514	12	3'-9"	Bt.	47
T603	2	12'-8"	Bt.	38	T401	29	11'-6"	St.	223	B515	3	19'-4"	St.	60
B501	12	4'-0"	Bt.	50	B701	87	18'-1"	Bt.	3216	B516	10	6'-5"	St.	67
B502	15	4'-0"	Bt.	63	B702	87	11'-6"	St.	2045	T501	39	7'-4"	Bt.	298
B503	2	9'-9"	Bt.	20	T601	87	13'-10"	Bt.	1808	T502	10	6'-3"	St.	65
B504	1	9'-10"	Bt.	10	T602	87	11'-6"	St.	1503	T508	4	6'-2"	Bt.	26
B505	2	10'-0"	Bt.	21	B501	72	4'-0"	Bt.	300	T509	4	5'-0"	Bt.	21
B506	1	10'-1"	Bt.	11	T501	348	7'-4"	Bt.	2662	T521	6	18'-2"	Bt.	114
B507	2	10'-3"	Bt.	21	T502	72	6'-3"	St.	469	T522	6	18'-4"	Bt.	115
B508	2	10'-4"	Bt.	22	T503	138	27'-8"	St.	3982	T523	6	16'-3"	Bt.	102
B509	1	9'-7"	Bt.	10	T506	24	5'-2"	Bt.	129	T524	6	16'-5"	Bt.	103
B510	1	8'-10"	Bt.	9	T507	24	3'-9"	Bt.	94	T525	11	18'-0"	St.	207
B511	6	22'-6"	St.	141	T508	12	6'-2"	Bt.	77	T526	11	24'-0"	St.	275
B512	6	21'-4"	St.	134	T509	12	5'-0"	Bt.	63	T527	6	22'-8"	St.	142
B513	6	3'-2"	Bt.	20	B401	87	16'-11"	Bt.	383	T528	1	14'-10"	Bt.	15
T501	54	7'-4"	Bt.	413	T401	87	11'-6"	St.	668	T529	1	15'-8"	Bt.	16
T502	12	6'-3"	St.	78	B601	16	18'-2"	Bt.	437	T530	4	16'-7"	St.	69
T506	8	5'-2"	Bt.	43	B602	15	11'-6"	St.	259	T531	2	13'-7"	Bt.	28
T507	8	3'-9"	Bt.	31	T601	16	13'-10"	Bt.	332	T532	2	10'-7"	Bt.	22
T510	15	16'-7"	St.	259	T602	15	11'-6"	St.	259	T533	10	7'-9"	Bt.	81
T511	15	14'-2"	St.	222	B501	14	4'-0"	Bt.	59	T535	6	6'-5"	St.	40
T512	4	14'-10"	Bt.	62	T501	62	7'-4"	Bt.	474	T536	2	6'-1"	Bt.	13
T513	4	15'-9"	Bt.	66	T502	14	6'-3"	St.	91	T537	6	4'-8"	Bt.	29
T514	4	16'-8"	Bt.	70	T505	46	15'-8"	St.	752	T538	2	4'-4"	Bt.	9
T515	4	15'-9"	Bt.	66	T506	8	5'-2"	Bt.	43	T539	24	3'-0"	Bt.	75
T516	10	2'-8"	Bt.	28	T507	8	3'-9"	Bt.	31	T540	6	2'-8"	St.	17
T517	10	21'-8"	St.	226	T508	4	6'-2"	Bt.	26	T541	3	5'-10"	Bt.	18
T518	10	11'-0"	St.	115	T509	4	5'-0"	Bt.	21	T542	3	4'-6"	Bt.	14
T519	2	23'-4"	Bt.	49	B401	15	16'-11"	Bt.	170	T543	3	4'-5"	Bt.	14
T520	2	21'-4"	Bt.	45	T401	15	11'-6"	St.	115	T544	3	4'-10"	Bt.	15
B401	13	16'-11"	Bt.	147										
T401	13	11'-6"	St.	100										

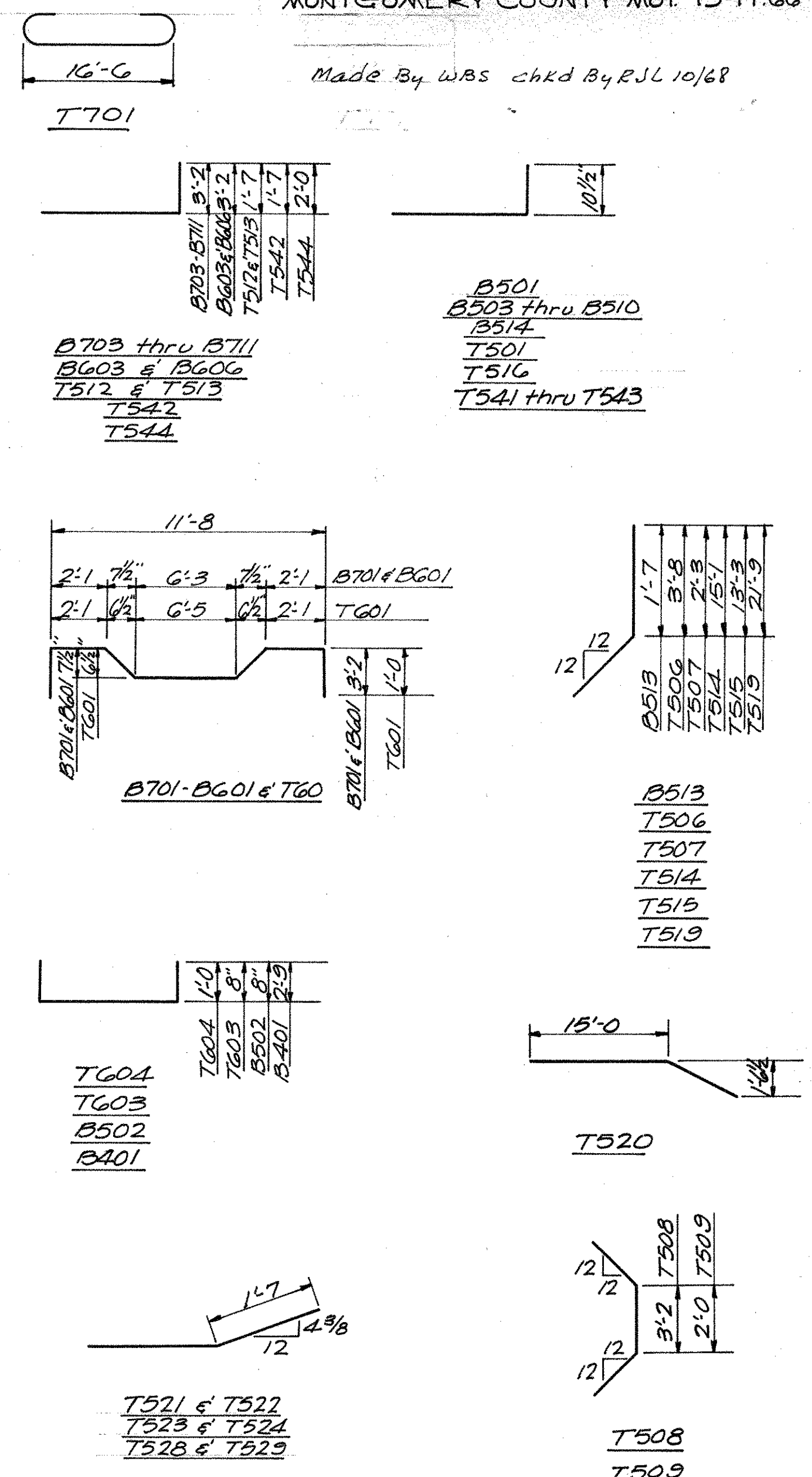
* Varies 10'-5" to 2'-11"
 # Varies 10'-3" to 2'-9"

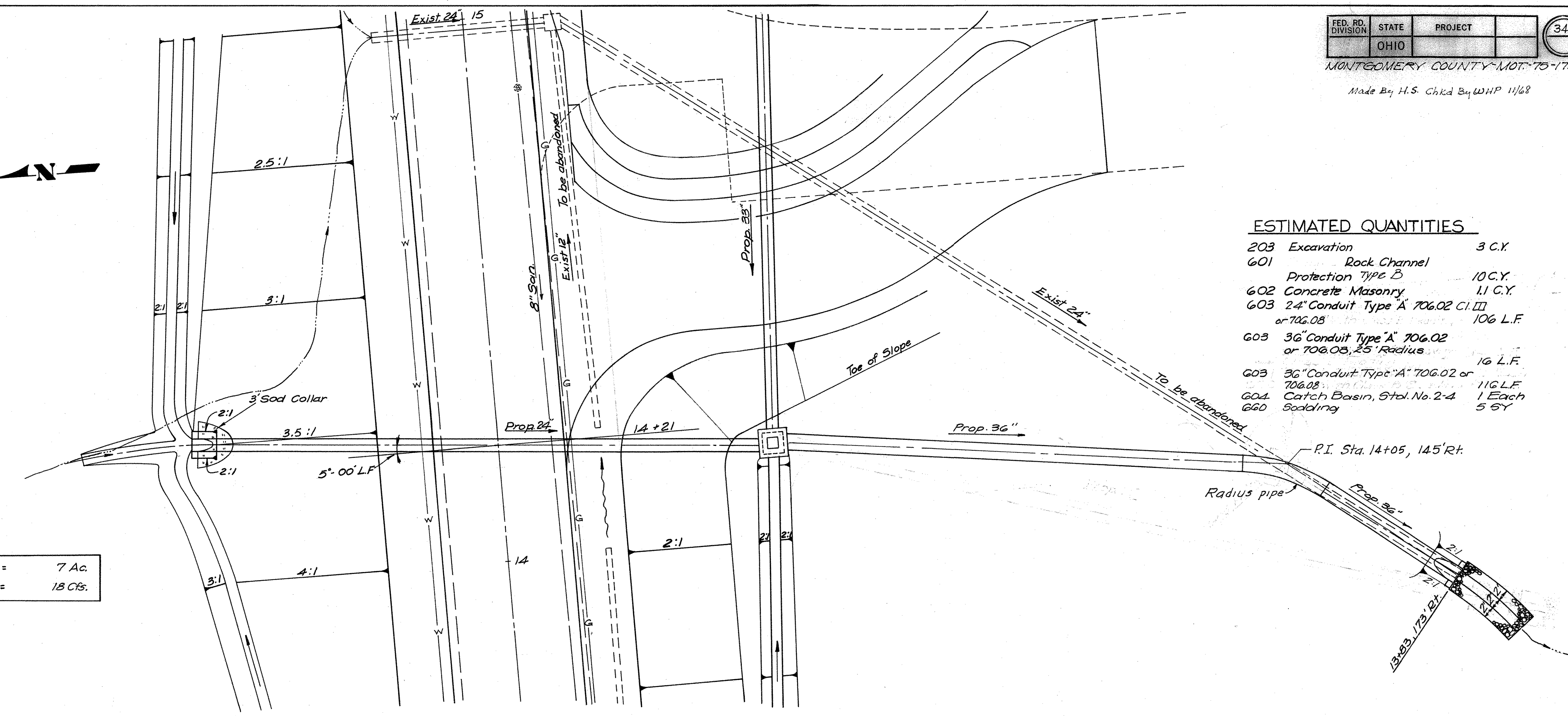
CHANNEL EARTHWORK

STATION	LENGTH	END AREA (Sq. Ft.)		VOLUME (Cu. Yds.)	
		CUT	FILL	CUT	FILL
0+00		0	0	0	10
0+80	80'	0	7	12	7
1+00	20'	32	12	25	10
1+20	20'	38	16	36	14
1+40	20'	60	21	50	11
1+60	20'	74	8		
3+20	23'	0	0	69	0
3+43	15'	161	0	70	0
3+58	8'	91	0	23	0
3+66		65	0		
TOTAL				284	52

ESTIMATED QUANTITIES

ITEM	DESCRIPTION	UNITS
202	Existing Structures Removed	Lump Sum
203	Excavation not including embankment construction	284 Cu. Yds.
503	Embankment	52 Cu. Yds.
503	Unclassified excavation	589 Cu. Yds.
509	Reinforcing steel	37,276 Lbs.
511	Class "C" concrete	268 Cu. Yds.
601	Rock Channel Protection Type B	41.8 Cu. Yds.
603	15" Conduit Type "B"	4 Lin. Ft.



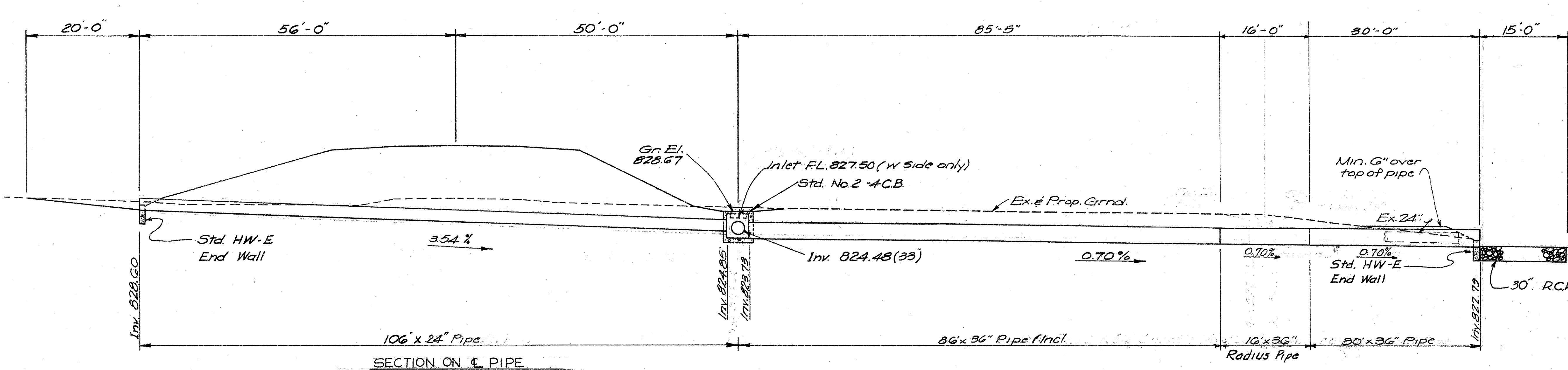


DA. = 7 Ac.
 Q25 = 13 Cfs.

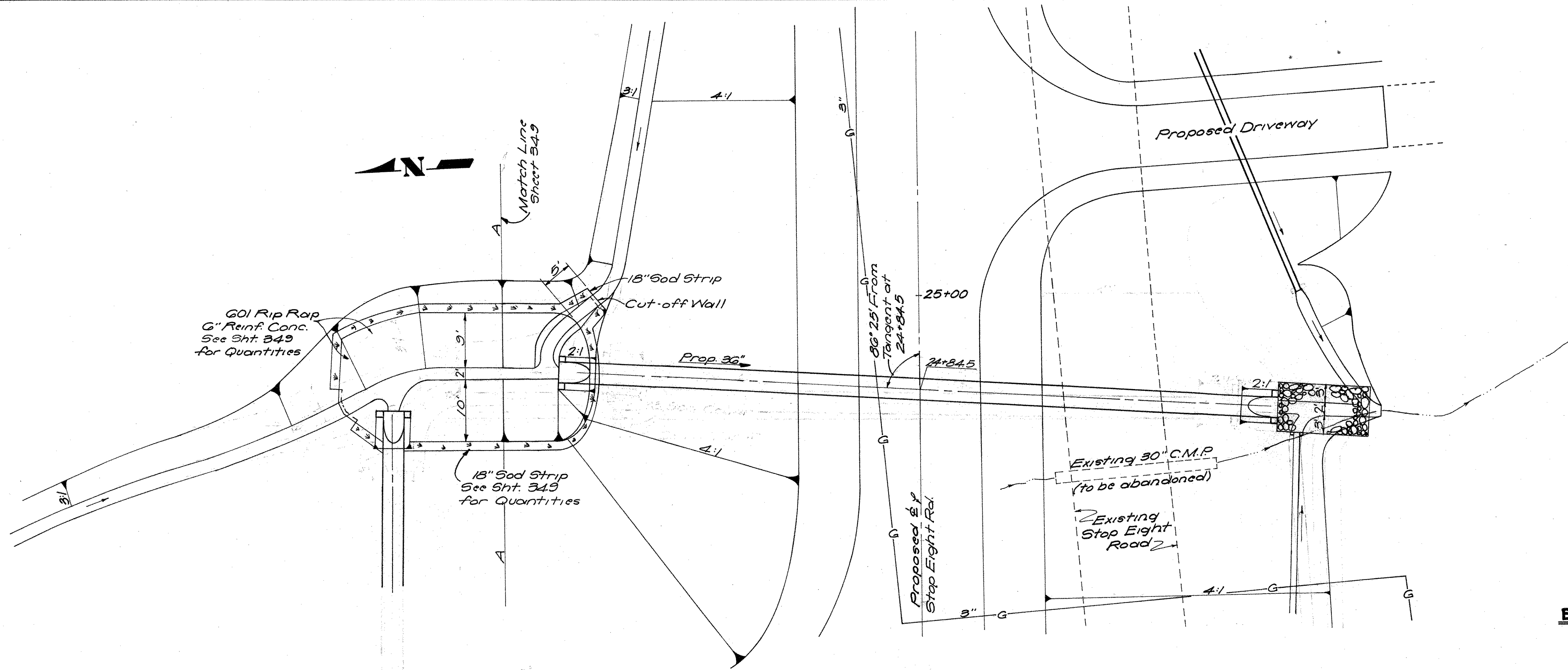
ESTIMATED QUANTITIES

203	Excavation	3 C.Y.
601	Rock Channel Protection Type B	10 C.Y.
602	Concrete Masonry	1.1 C.Y.
603	24" Conduit Type A 706.02 Cl. III or 706.08	106 L.F.
603	36" Conduit Type A 706.02 or 706.08, 25' Radius	16 L.F.
603	36" Conduit Type A 706.02 or 706.08	116 L.F.
604	Catch Basin, Std. No. 2-4	1 Each
660	Sodding	5 Sq.

PLAN
 Scale 1" = 10'



SECTION ON PIPE

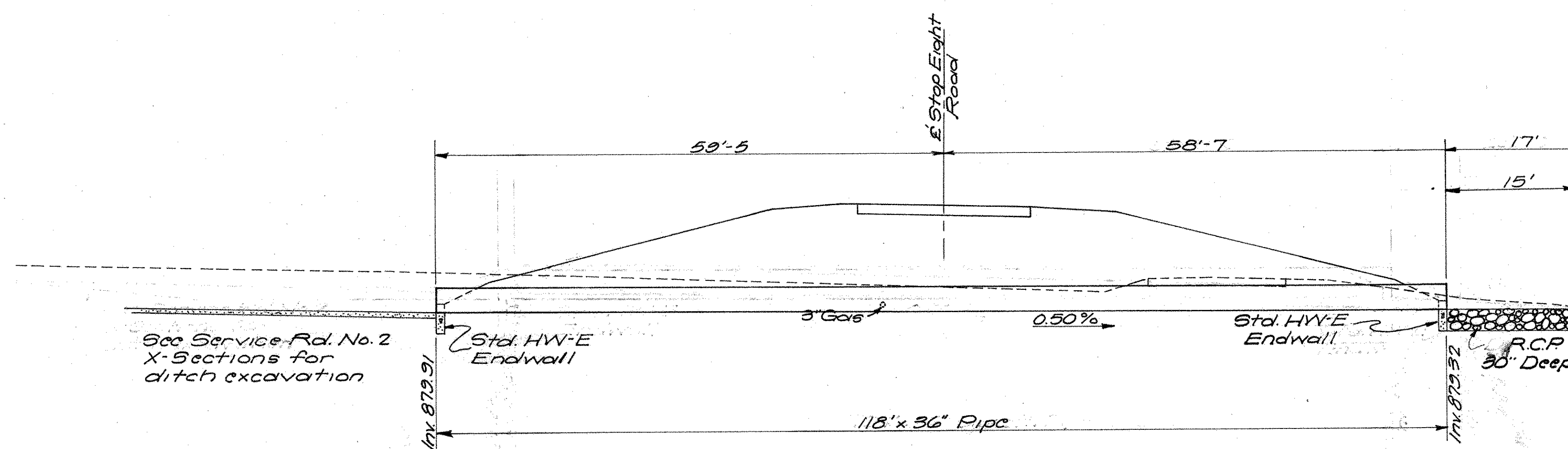


PLAN
 Scale 1"=10'

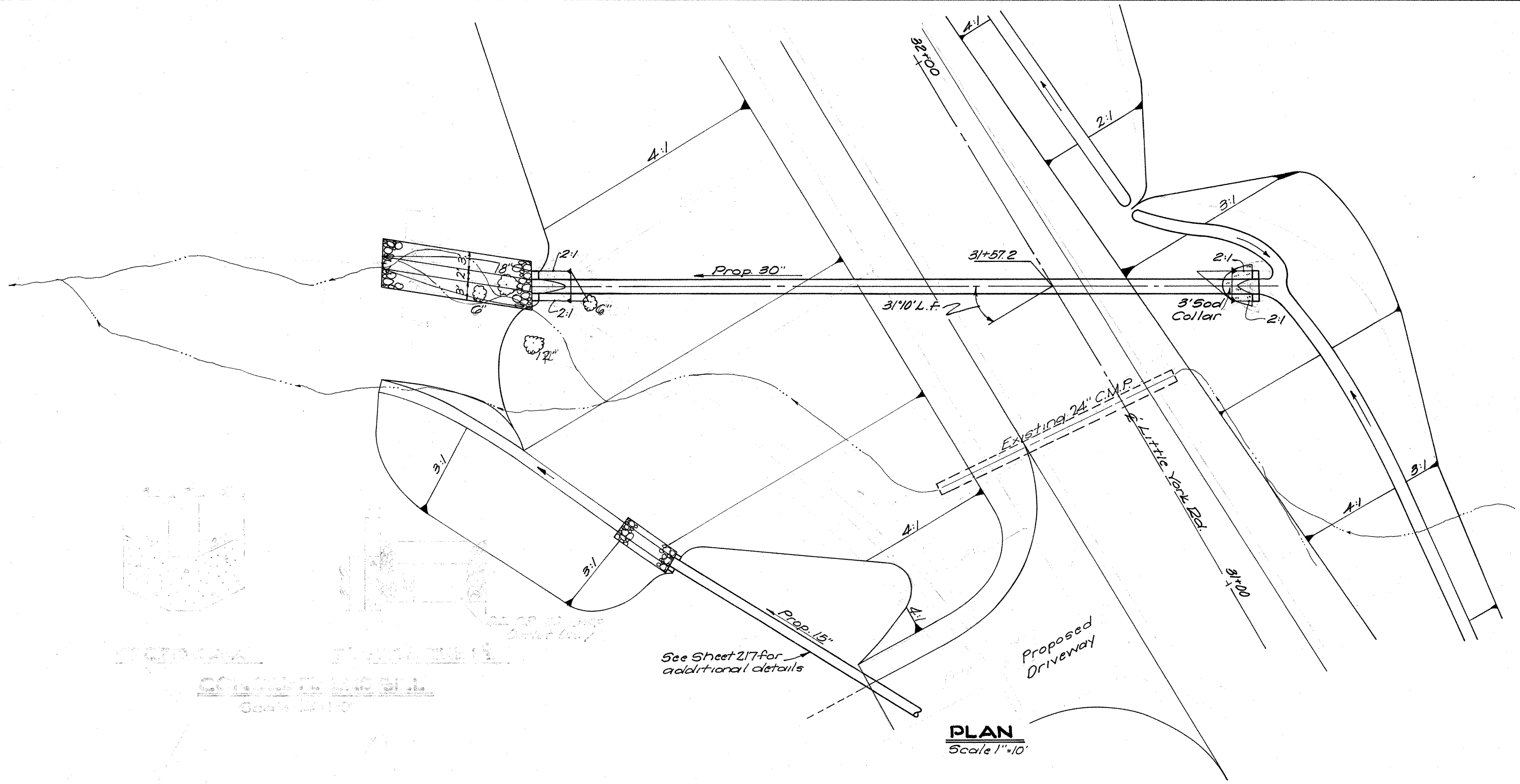
ESTIMATED QUANTITIES

203	Excavation	1 Cu. Yd.
G01	Type B Rock Channel Protection	130 Cu. Yds.
G02	Concrete Masonry	1.2 Cu. Yds.
G03	36" Conduit, Type "A", 706.02 or 706.0B with Class "B" Bedding	118 Lin. Ft.

D.A.	42 Ac.
Q25	46 c.f.s.



SECTION ALONG PIPE

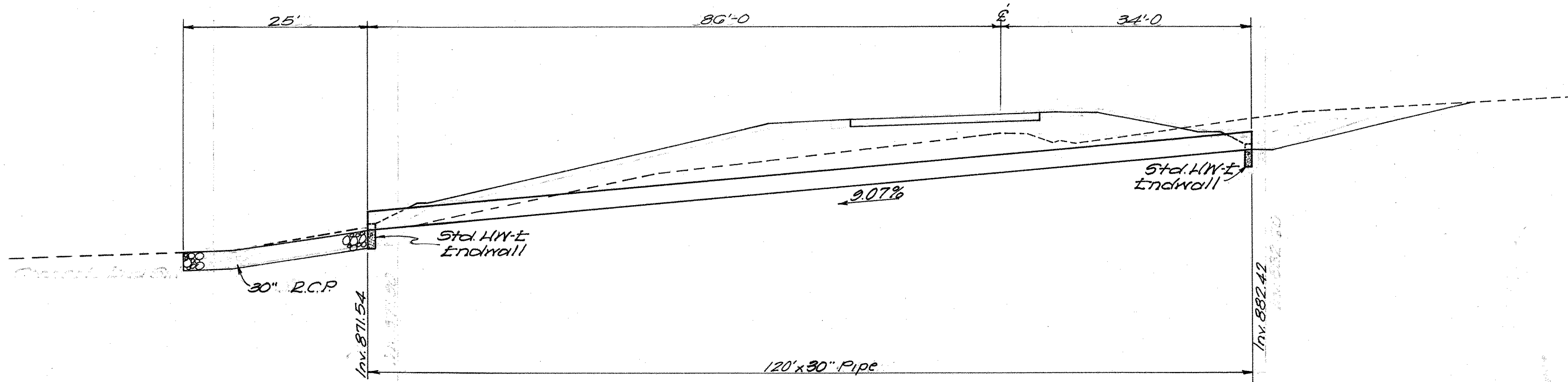


ESTIMATED QUANTITIES

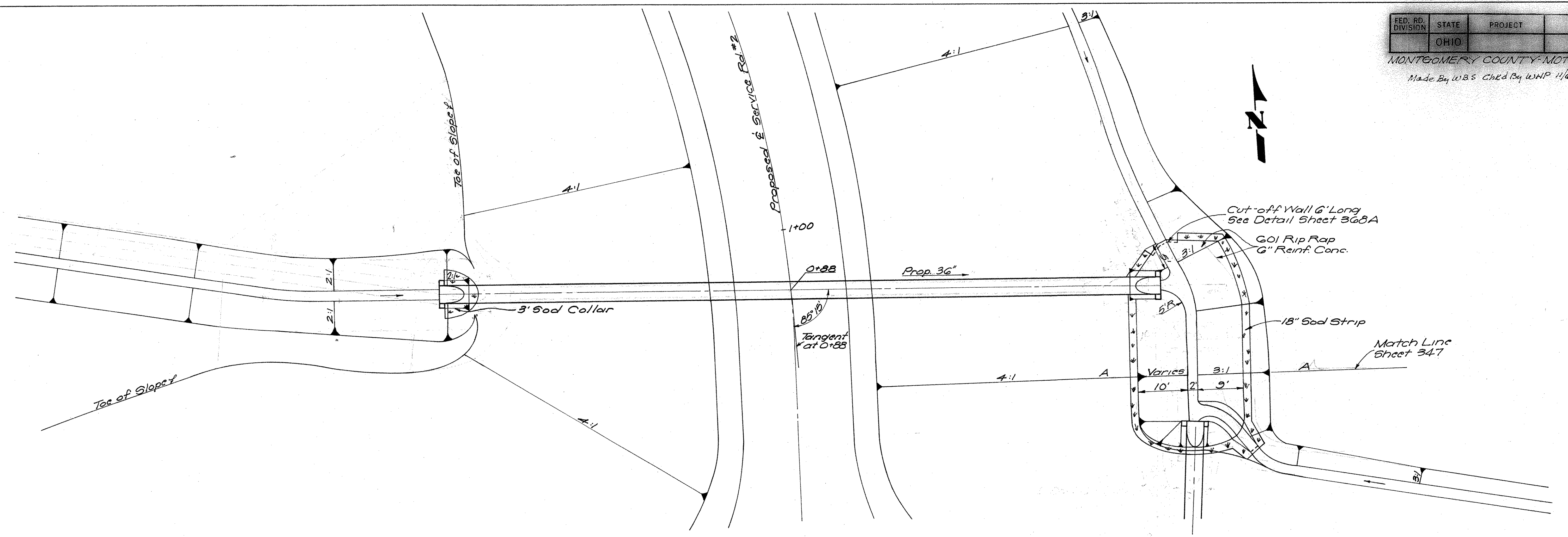
203 - Excavation	2 C.Y.
G01 - Rock Channel	
Protection Type B	22.2 C.Y.
G02 - Concrete Masonry	2 C.Y.
G03 - 30" Conduit Type "A" 70705	120 L.F.
G60 - Sodding	66 Y.

D.A.	3Ac
Q25	34 cfs

PLAN
Scale 1"=10'



SECTION ON E PIPE

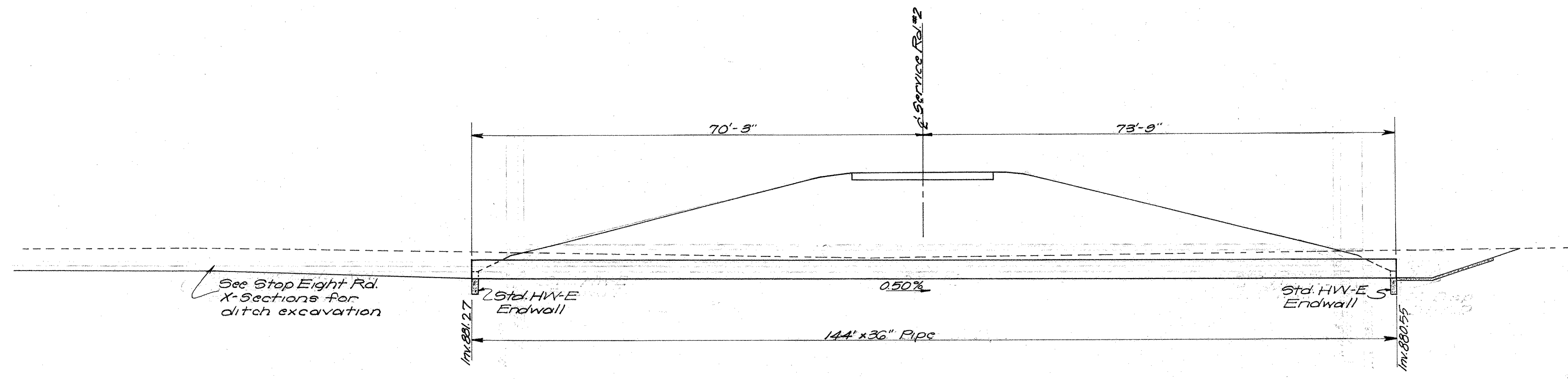


PLAN
 Scale 1"=10'

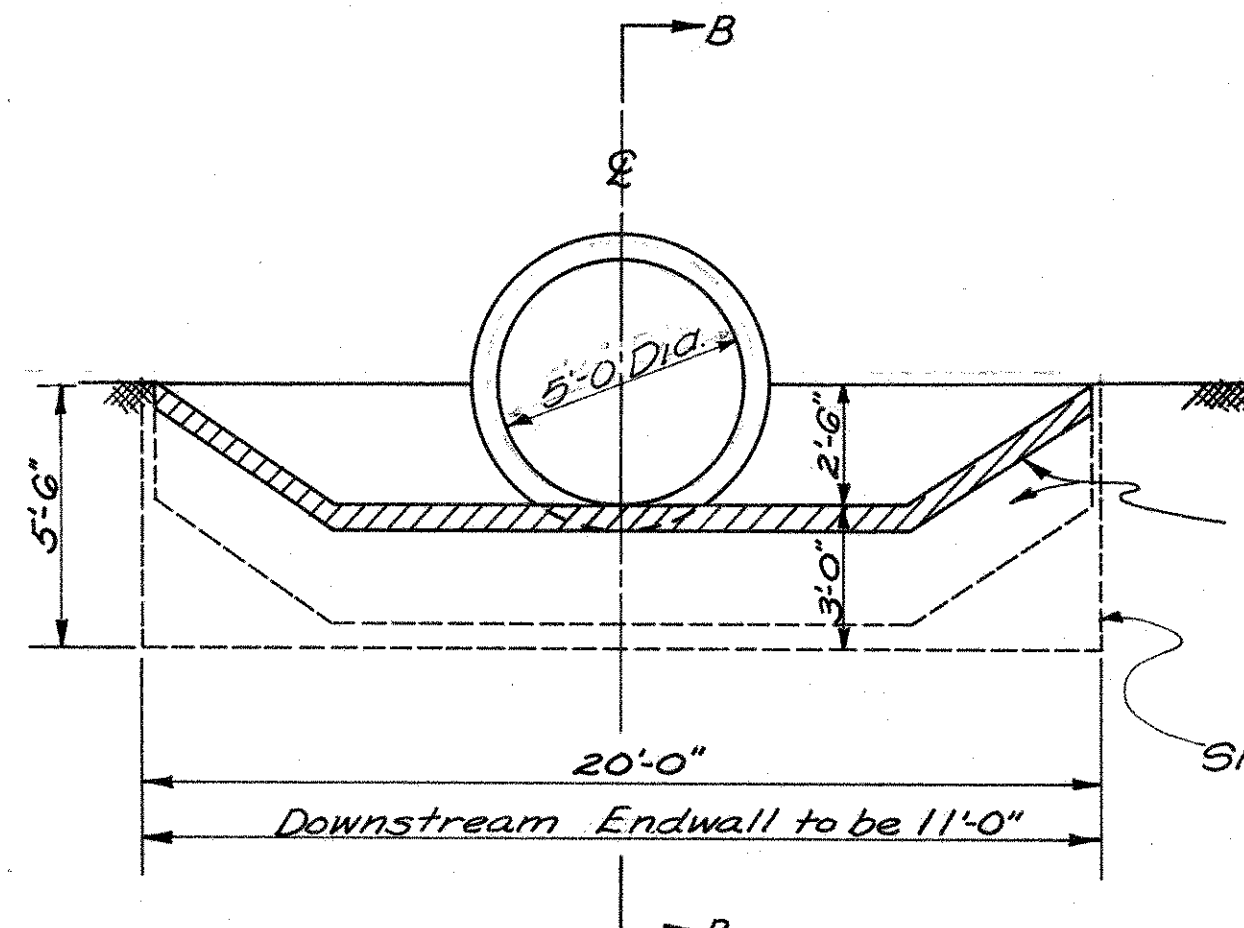
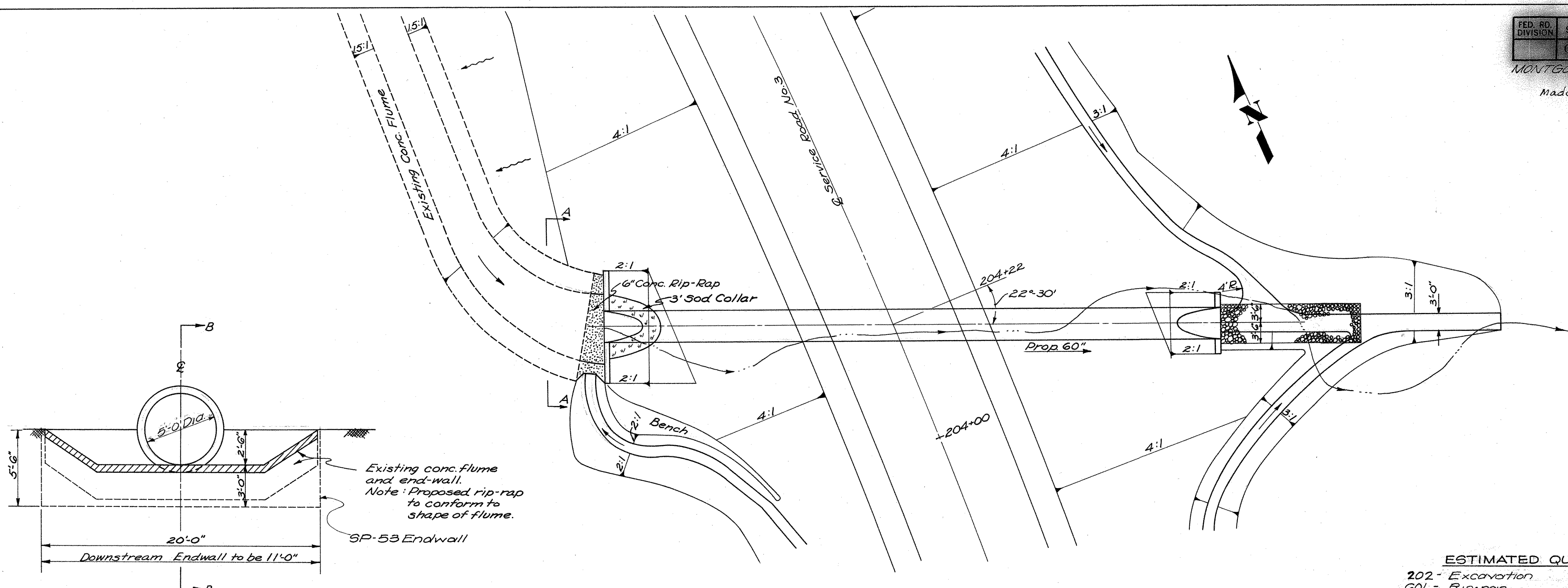
ESTIMATED QUANTITIES

203 Excavation	1 Cu. Yd.
G01 Rip Rap Using G" Reinf. Conc.	83 Sq. Yds.
G02 Concrete Masonry	12 Cu. Yd.
G03 36" Conduit, Type "A"; 706.02 C.I. III or 706.05	144 Lin. Ft.
G60 Sodding	23 Sq. Yds.

DA.	354c.
Q25	42 c. fs.



SECTION ALONG PIPE

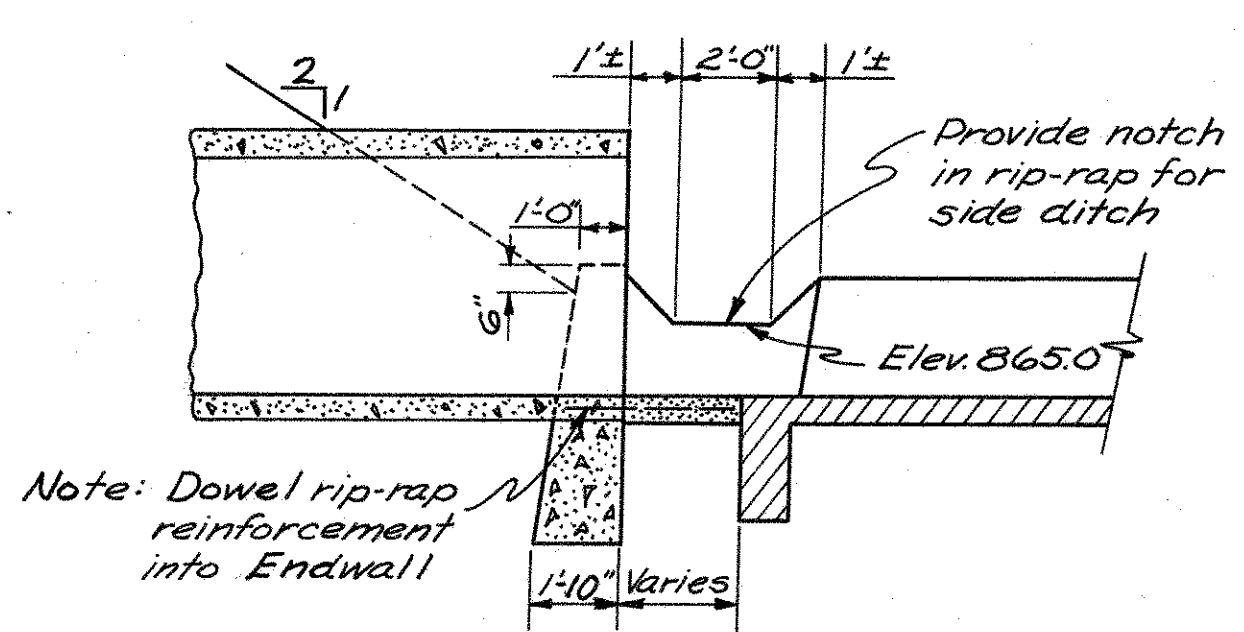


PLAN
Scale - 1" = 10'

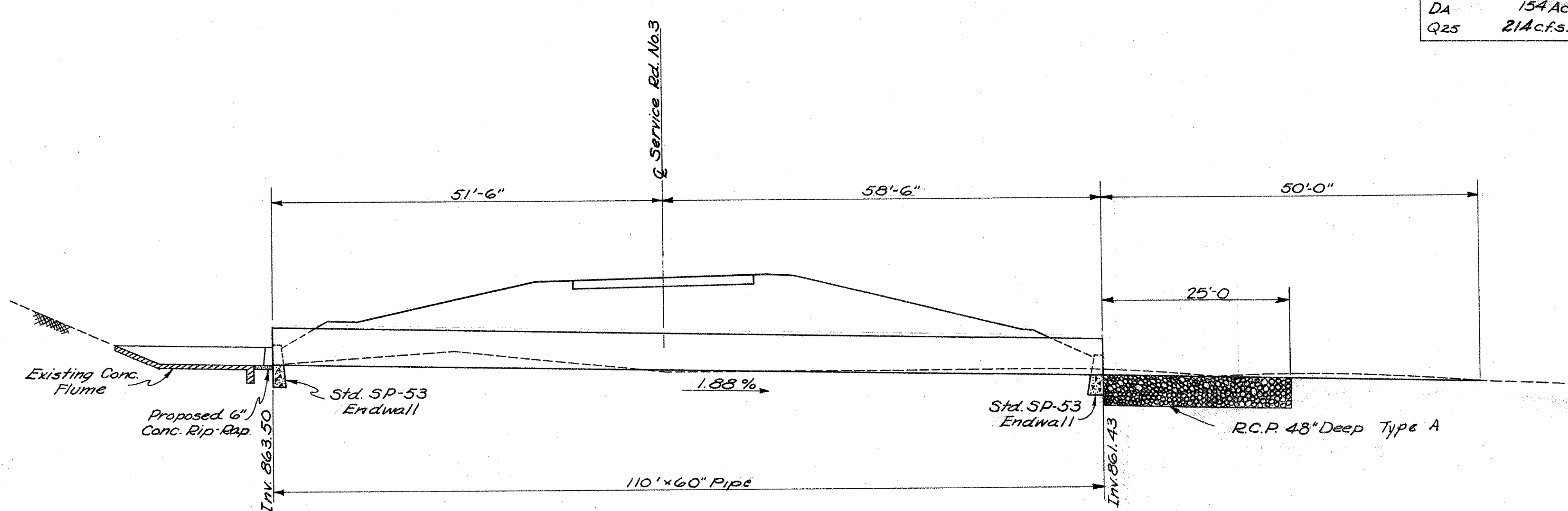
ESTIMATED QUANTITIES

202 - Excavation	3 Cu. Yds.
601 - Rip-rap	7 Sq. Yds.
601 - Type A Rock Channel Protection as per plan	29.2 Cu. Yds.
602 - Concrete Masonry	86 Cu. Yds.
603 - 60" Conduit, Type A, 706.02	110 Lin. Ft.
660 - Sodding	5 Sq. Yds.

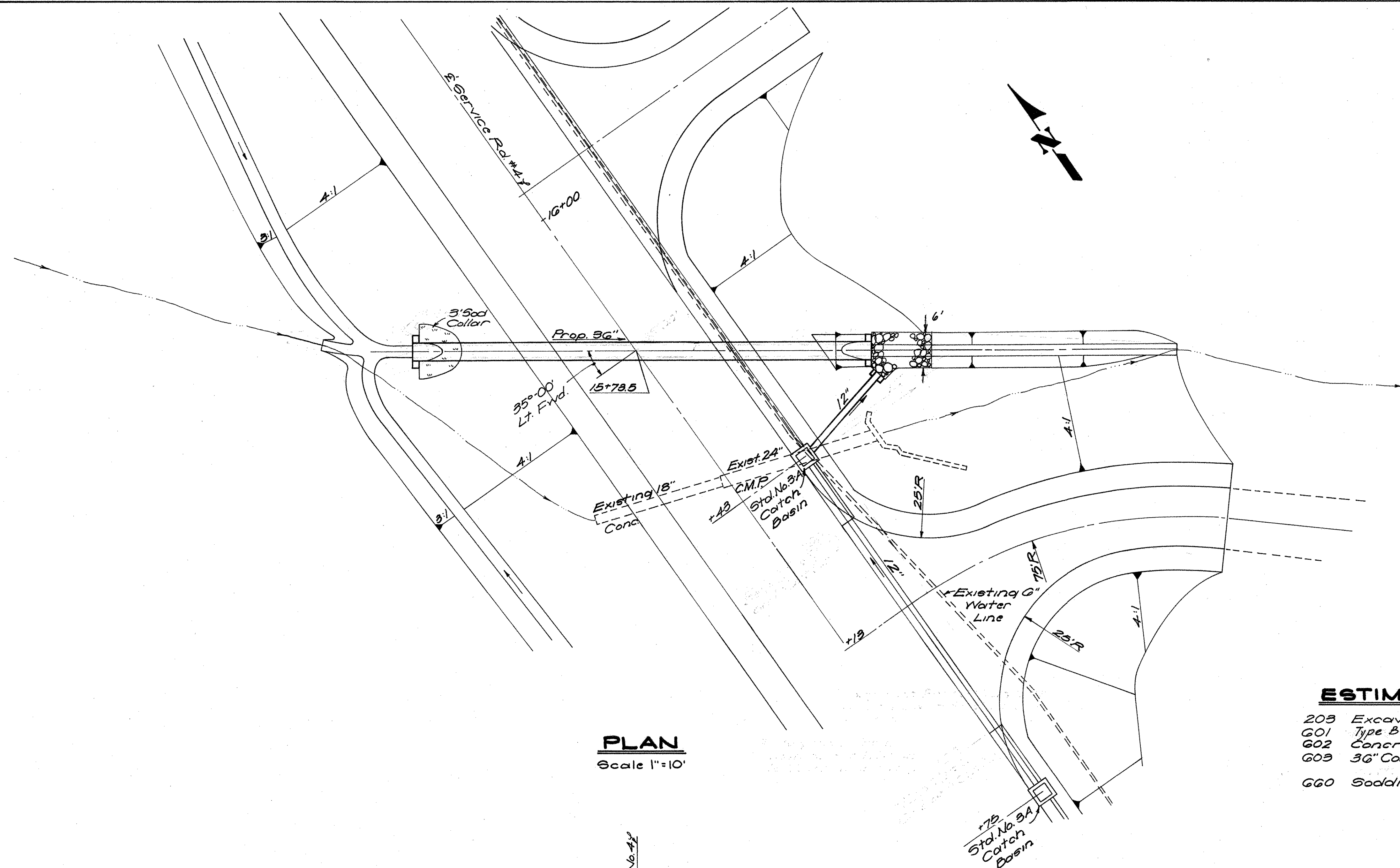
DA	154 Ac.
Q25	214 c.f.s.



SECTION B-B
Scale - 1/4" = 1'-0"



SECTION ALONG PIPE

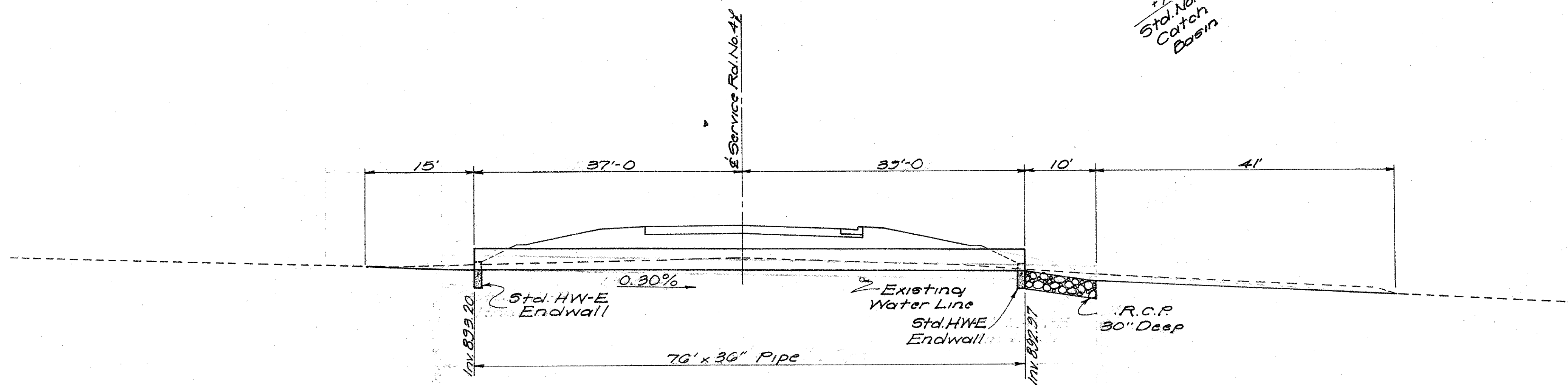


PLAN
 Scale 1"=10'

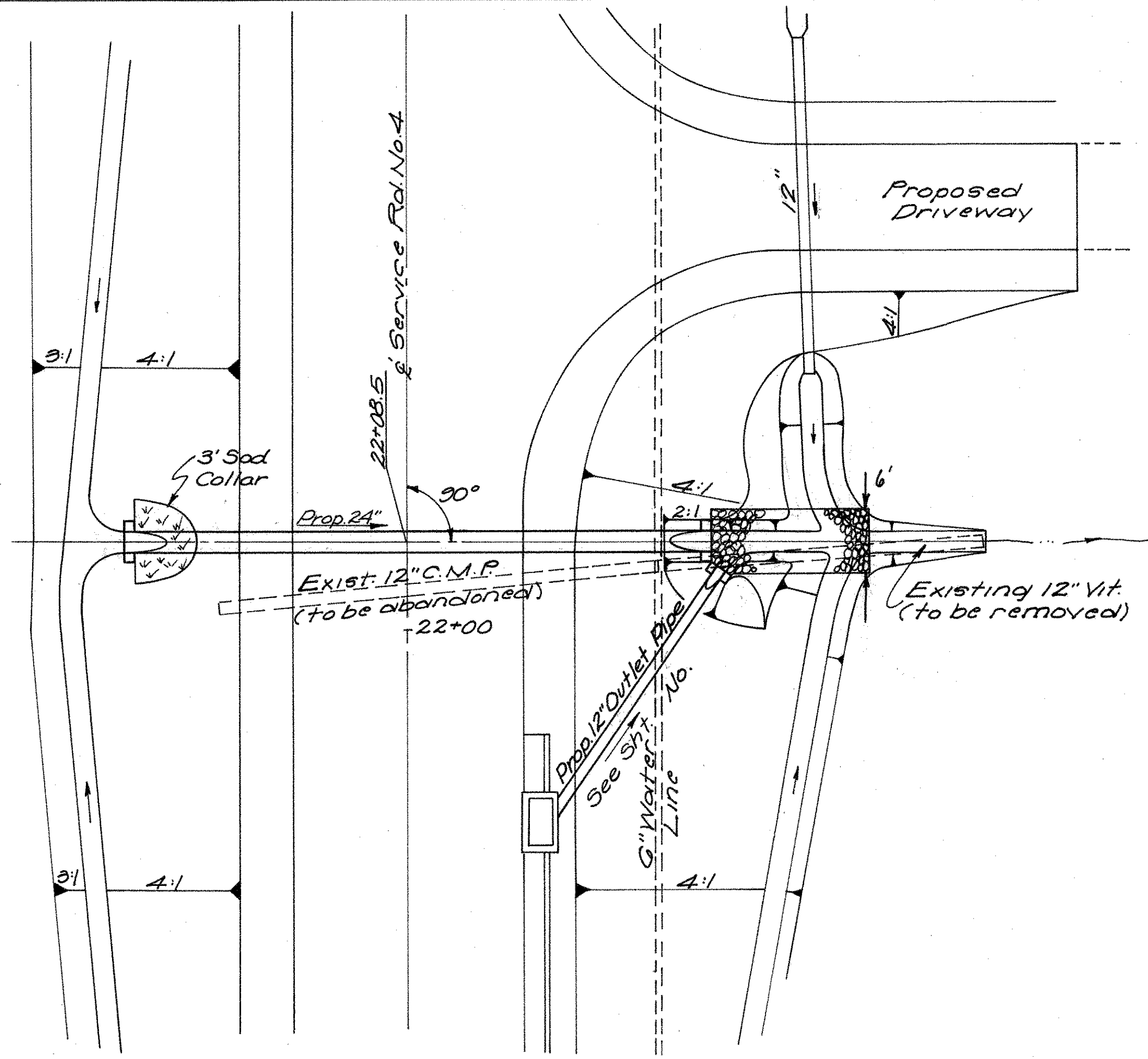
ESTIMATED QUANTITIES

203	Excavation	5 Cu. Yds.
G01	Type B Rock Channel Protection	6.7 Cu. Yds.
G02	Concrete Masonry	1.2 Cu. Yds.
G03	36" Conduit Type "A", 706.02 or 706.08	76 Lin. Ft.
G60	Sodding	4 Sq. Yds.

DA	25 Ac.
Q25	54 c.f.s.



SECTION ALONG E PIPE

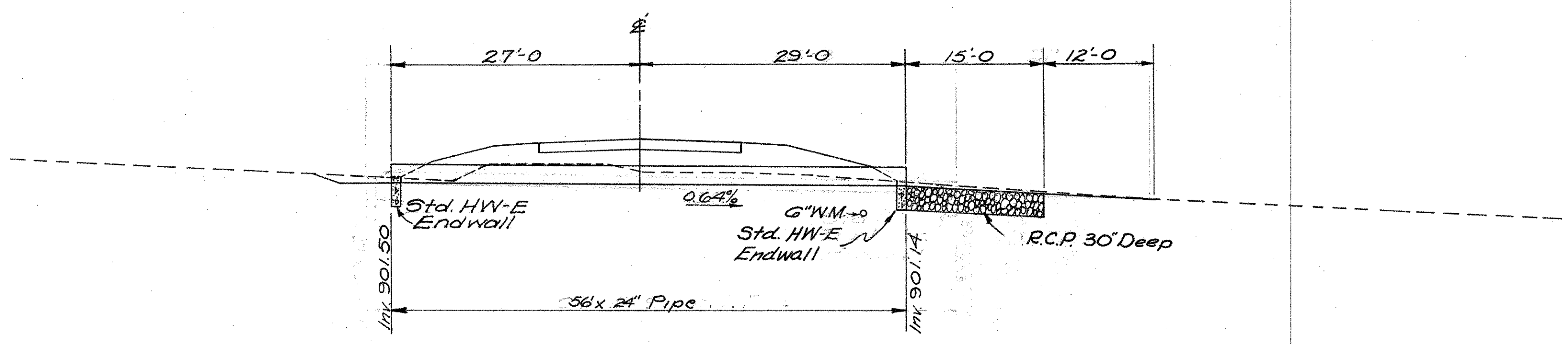


PLAN
 Scale 1"=10'

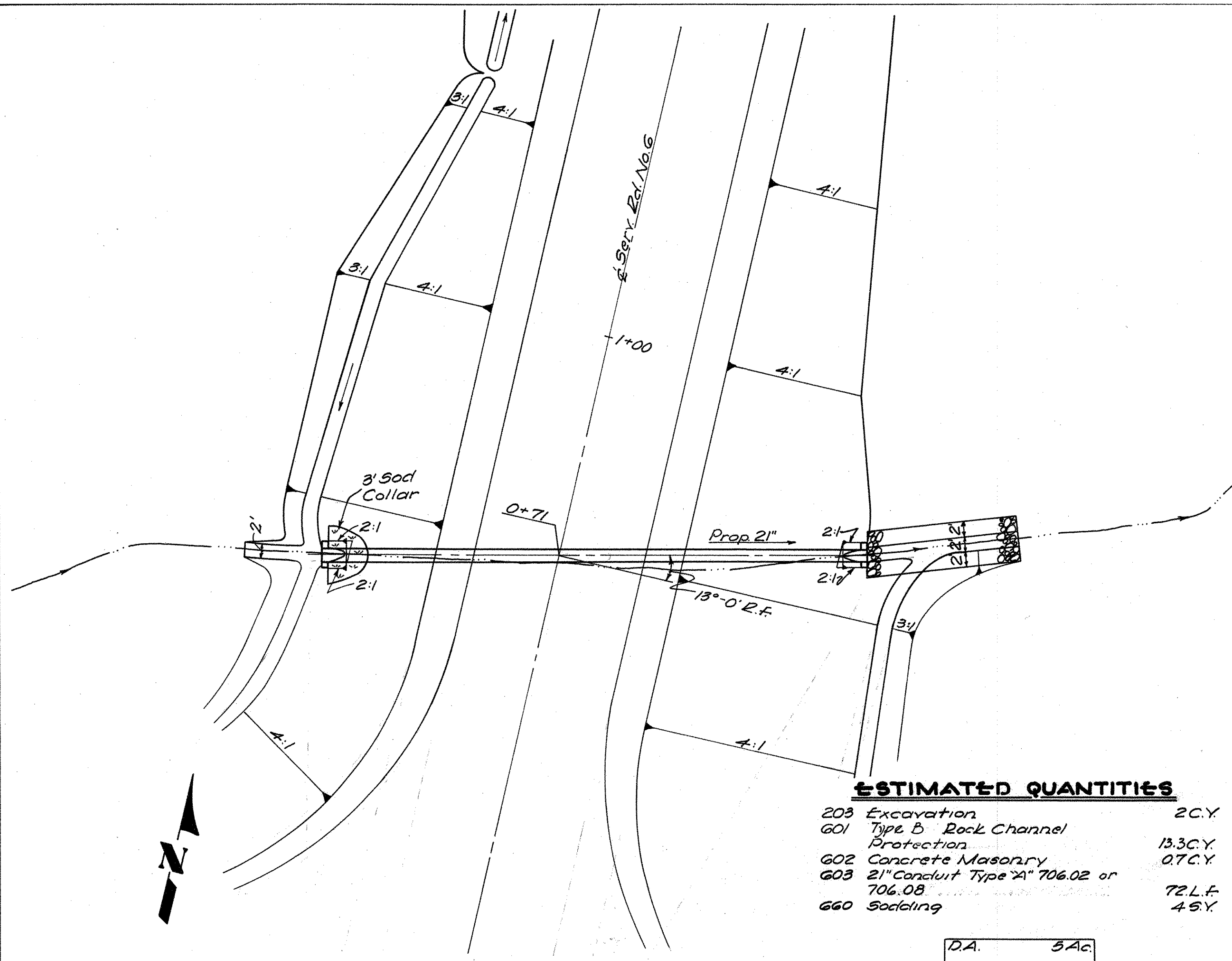
ESTIMATED QUANTITIES

202	Pipe Removed 24" & under	74 Lin. Ft.
203	Excavation	2 Cu. Yds.
G01	Type B Rock Channel Protection	10 Cu. Yds.
G02	Concrete Masonry	0.8 Cu. Yds.
G03	24" Conduit, Type "A", 706.02 or 706.08	56 Lin. Ft.
G60	Sodding	3 Sq. Yds.

DA.	7 Ac.
Q25	22 c.f.s.



SECTION ALONG & PIPE

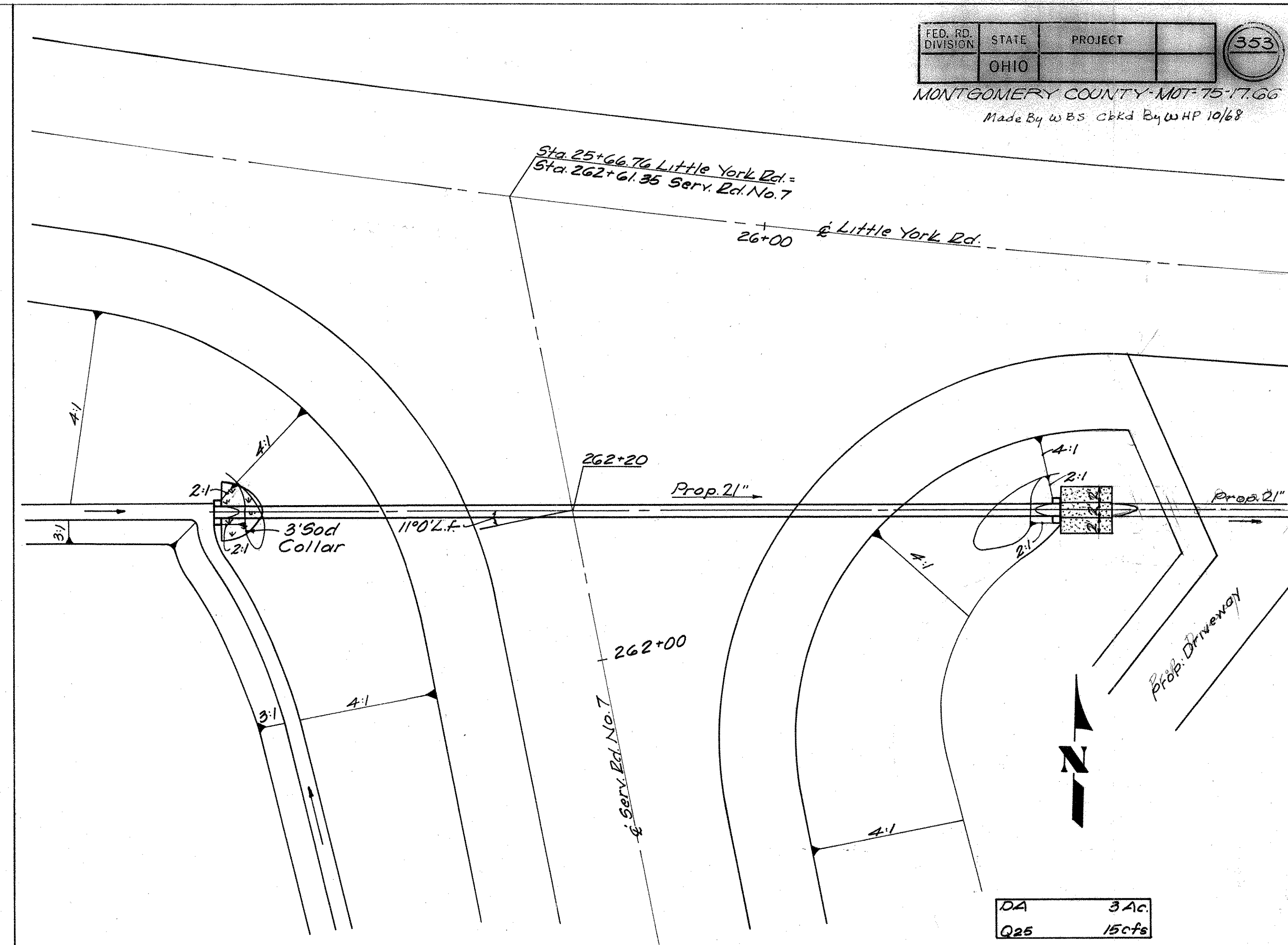


ESTIMATED QUANTITIES

203 Excavation	2 C.Y.
601 Type B Rock Channel Protection	13.3 C.Y.
602 Concrete Masonry	0.7 C.Y.
603 21" Conduit Type "A" 706.02 or 706.08	72 L.F.
660 Sodding	4.5 Y.

D.A.	5 Ac.
Q25	20 cfs

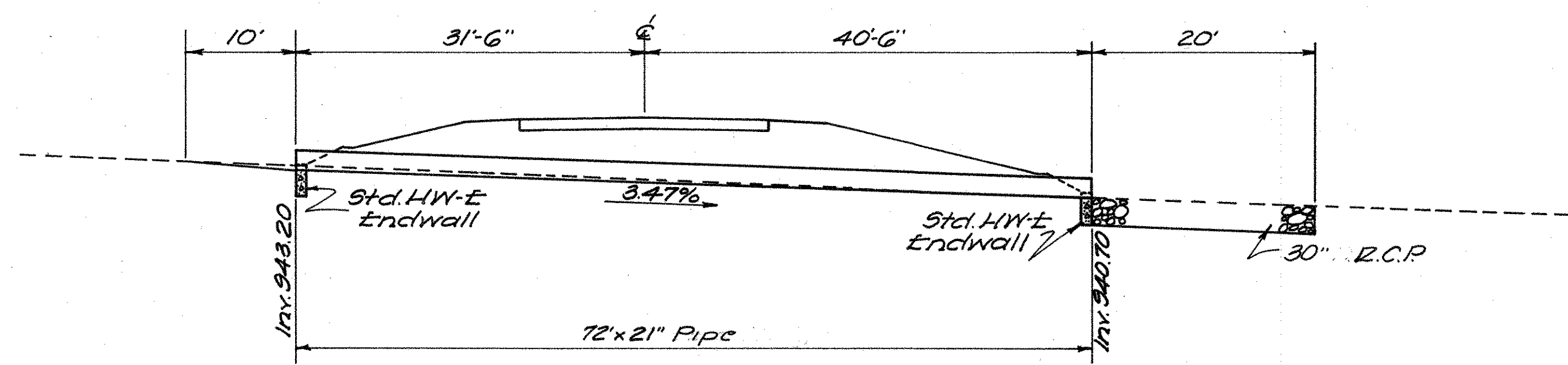
PLAN
Scale: 1"=10'



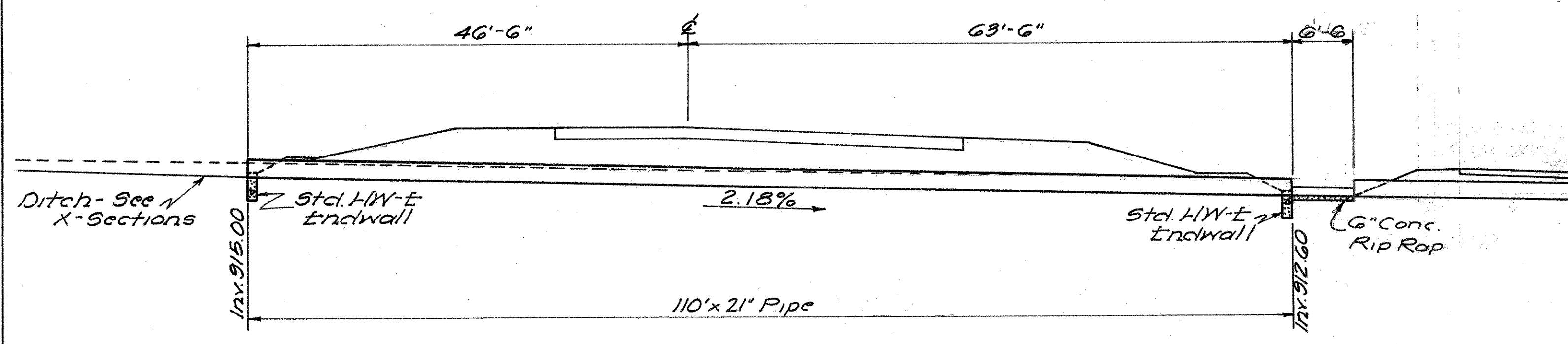
ESTIMATED QUANTITIES

601 Rip Rap using 6" Reinf. Conc.	5.5 Y.
602 Concrete Masonry	0.6 C.Y.
603 21" Conduit Type "A" 706.02 or 706.08	110 L.F.
660 Sodding	4.5 Y.

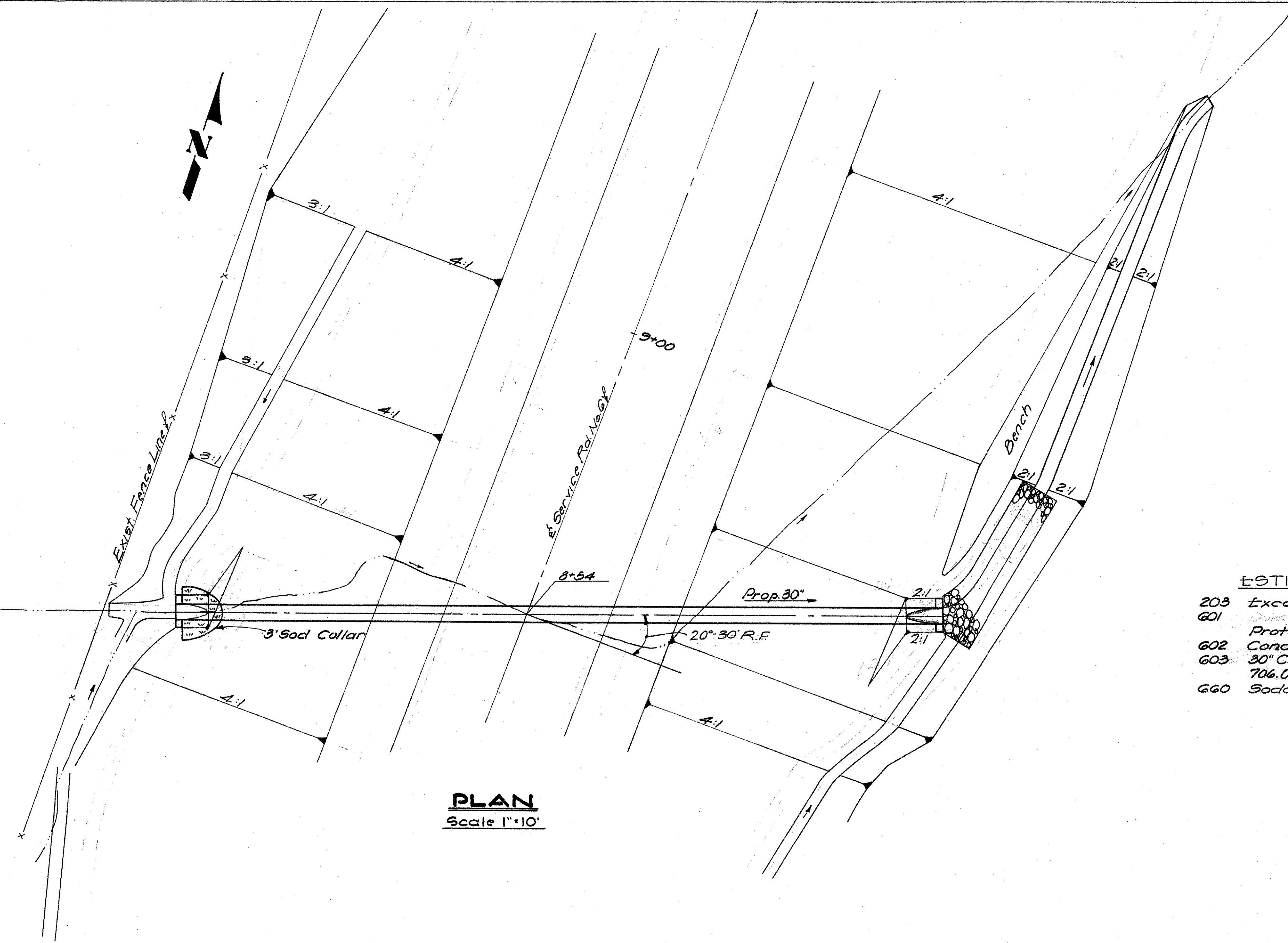
PLAN
Scale: 1"=10'



SECTION ON E PIPE



SECTION ON E PIPE

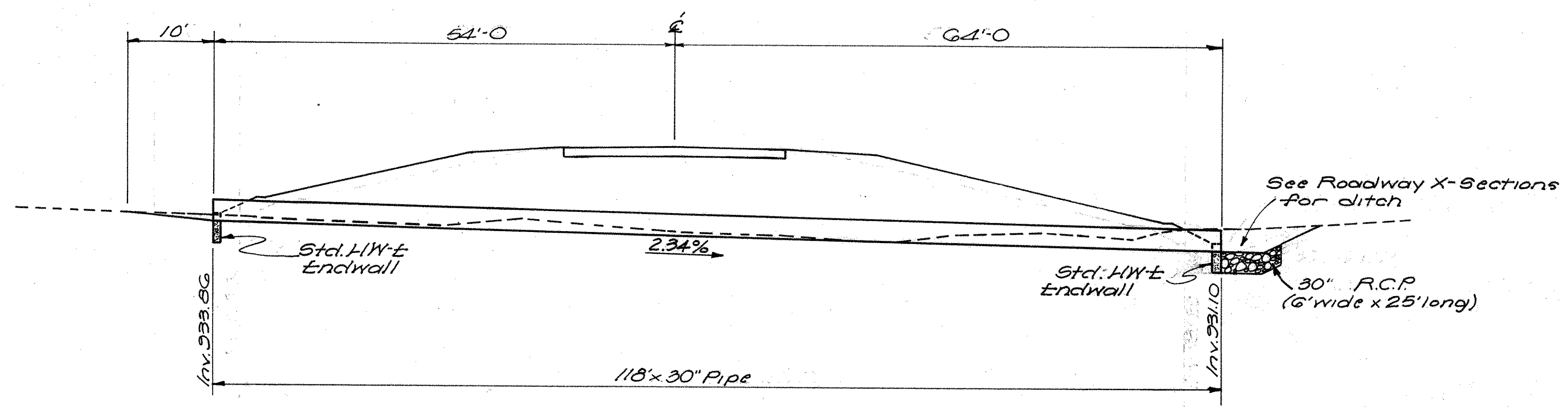


PLAN
Scale 1"=10'

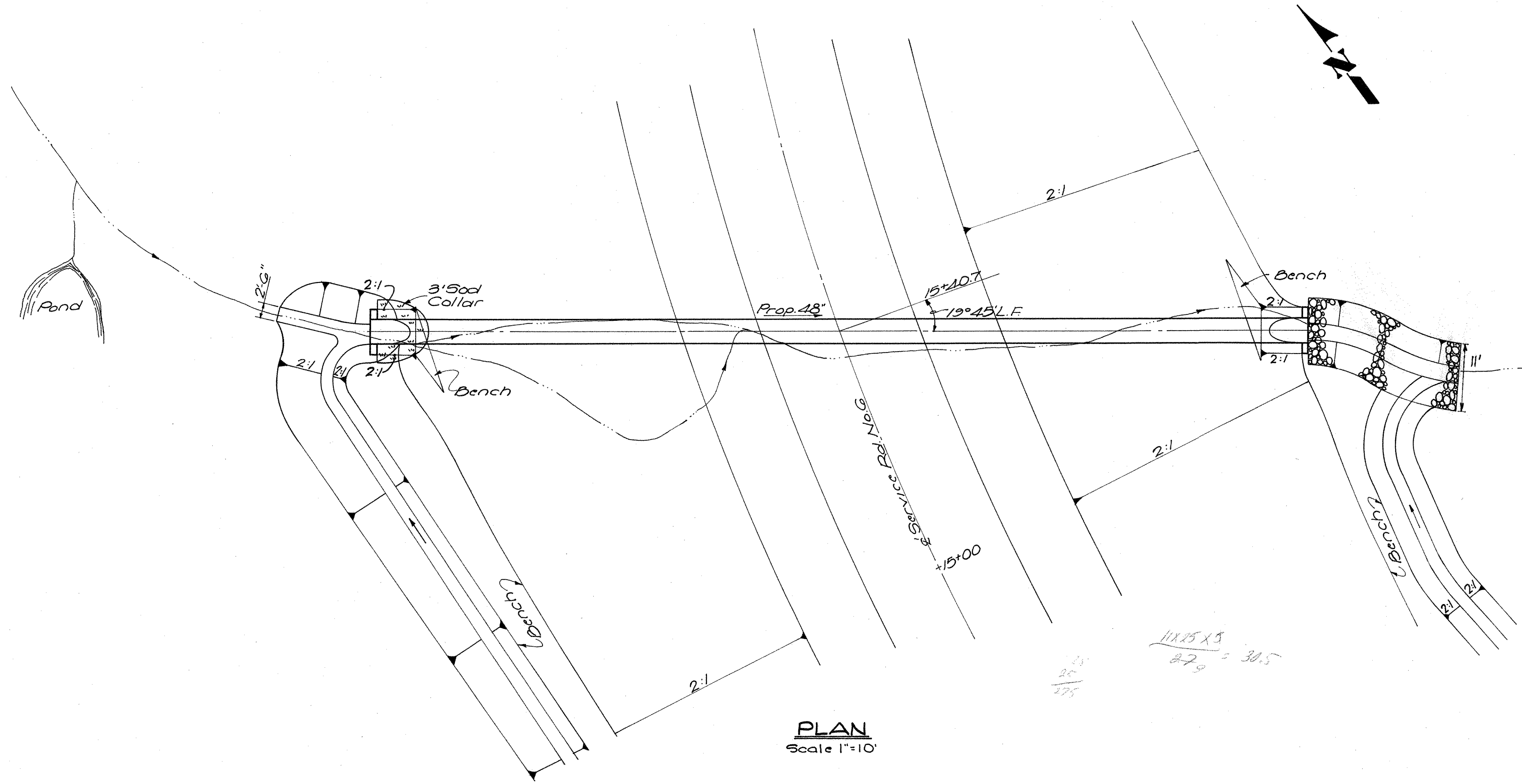
ESTIMATED QUANTITIES

203	Excavation	2 C.Y.
601	Rock Channel Protection	16.7 C.Y.
602	Concrete Masonry	1 C.Y.
603	30" Conduit Type 'A' 706.02 or 706.08	118 L.F.
660	Sodding	55 Y.

DA	16 Ac.
Q25	46cfs



SECTION ON 1/2 PIPE

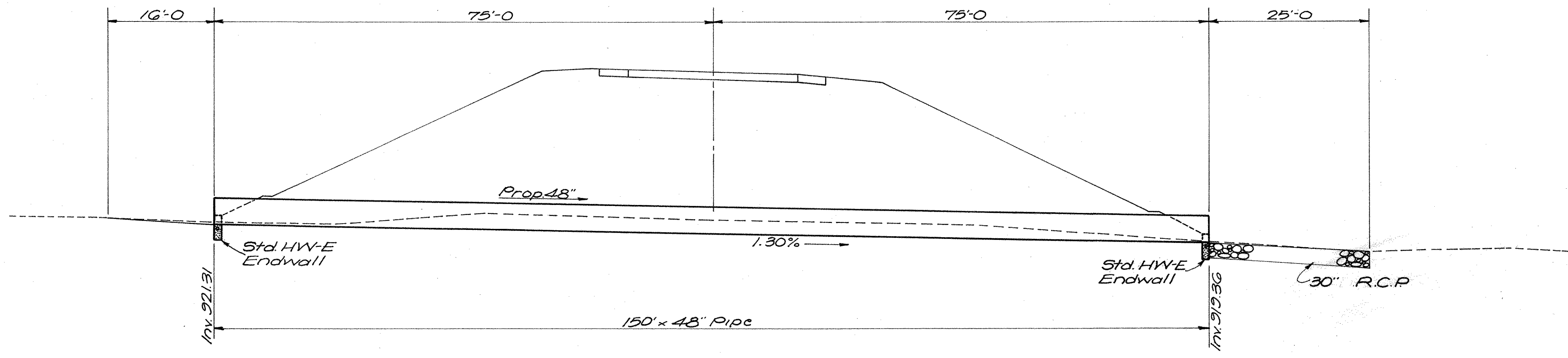


PLAN
Scale 1"=10'

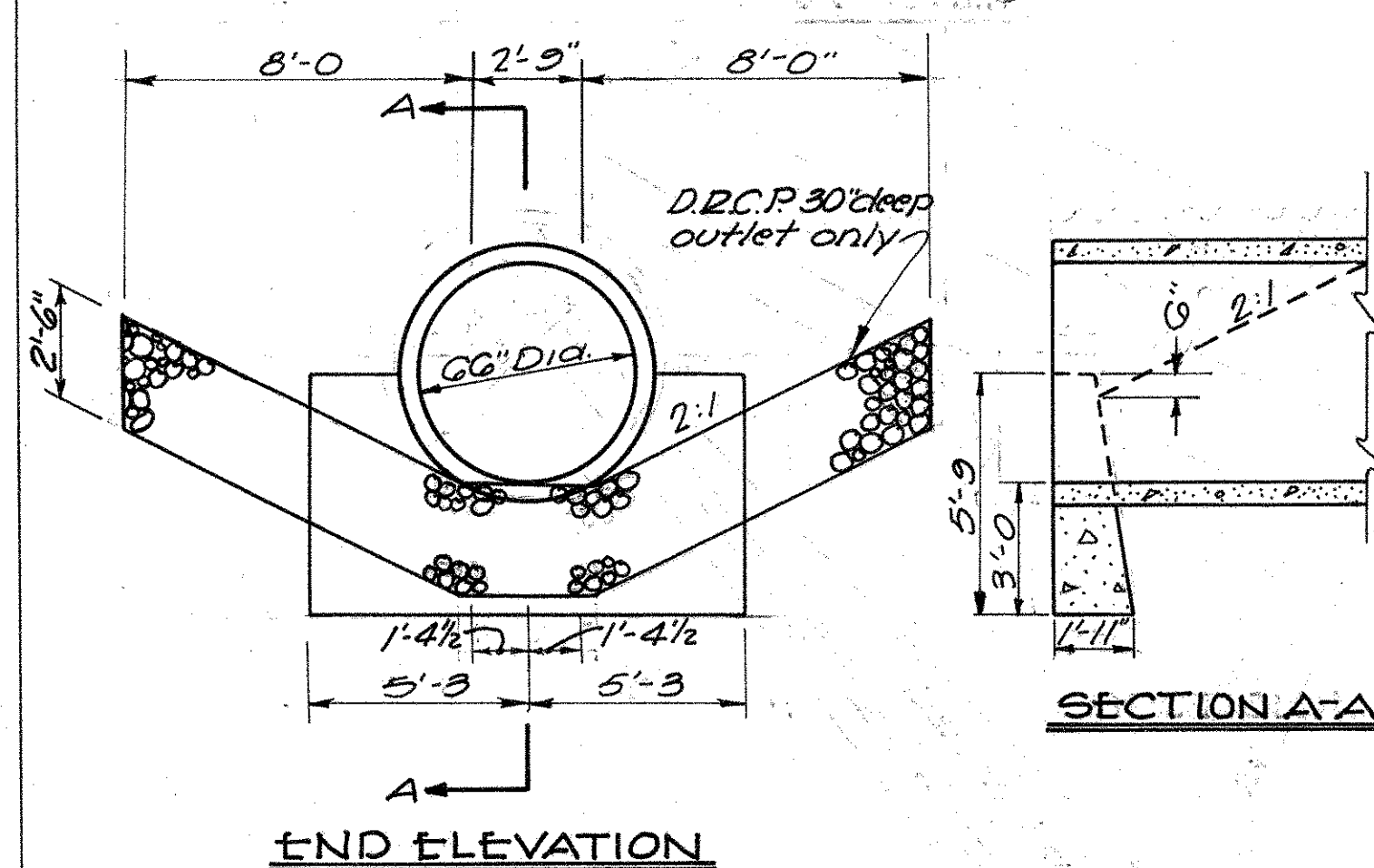
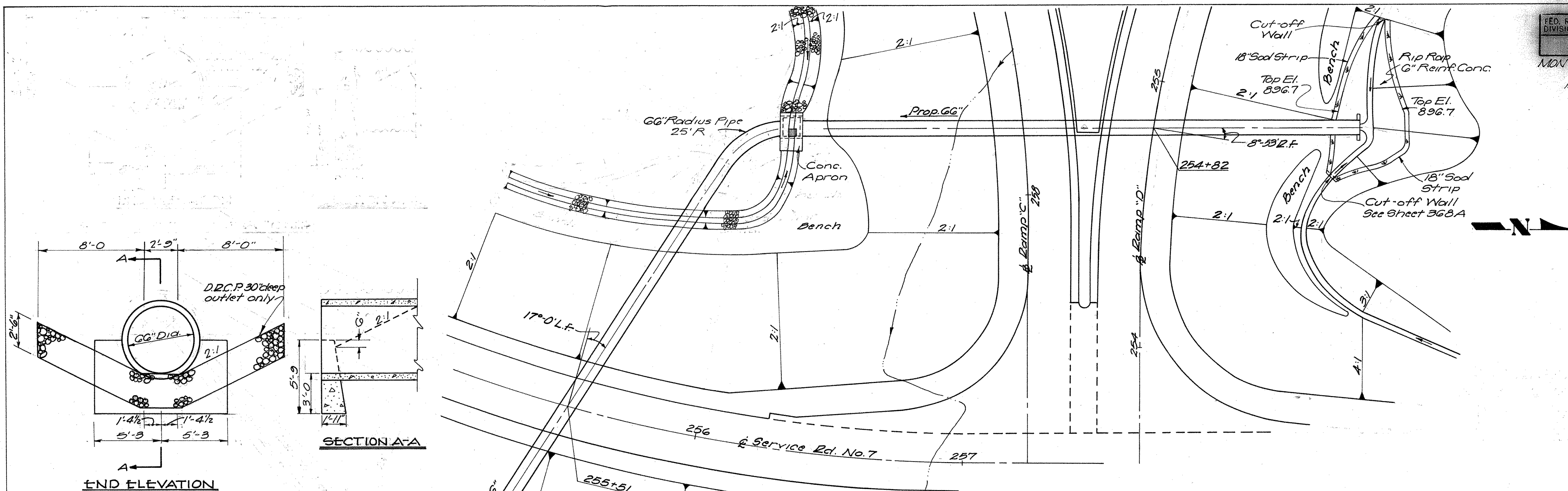
ESTIMATED QUANTITIES

203	Excavation	2 C.Y.
G01	Rock Channel Protection Type B	30.6 C.Y.
G02	Concrete Masonry	1.7 C.Y.
G03	48" Conduit, Type "A", 70G.02	150 L.F.
G60	Cl. IV with Class "B" Bedding Sodding	5 S.Y.

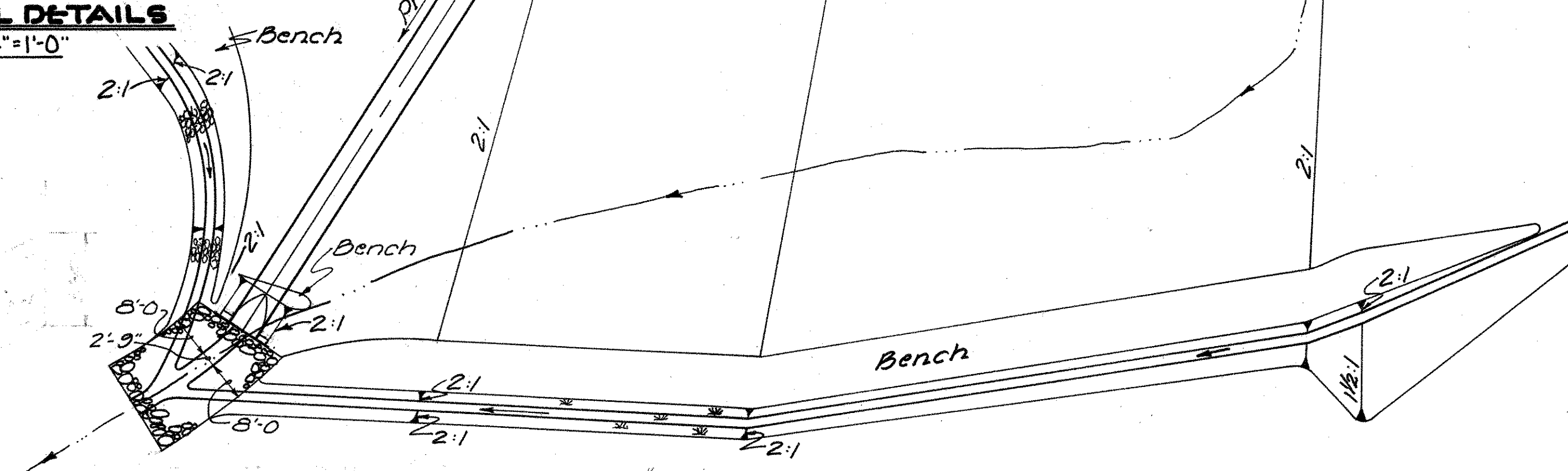
DA 10/Ac.
Q25 160cfs



SECTION ON PIPE



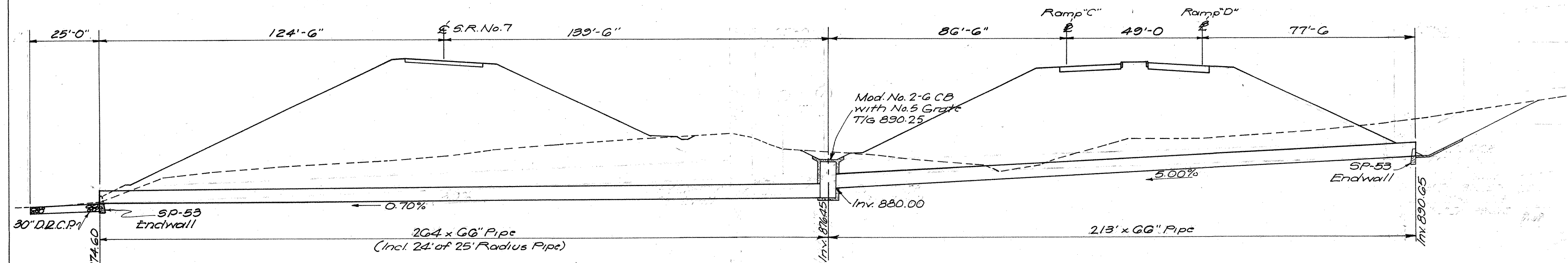
SP-53 ENDWALL DETAILS
Scale: 1/4" = 1'-0"



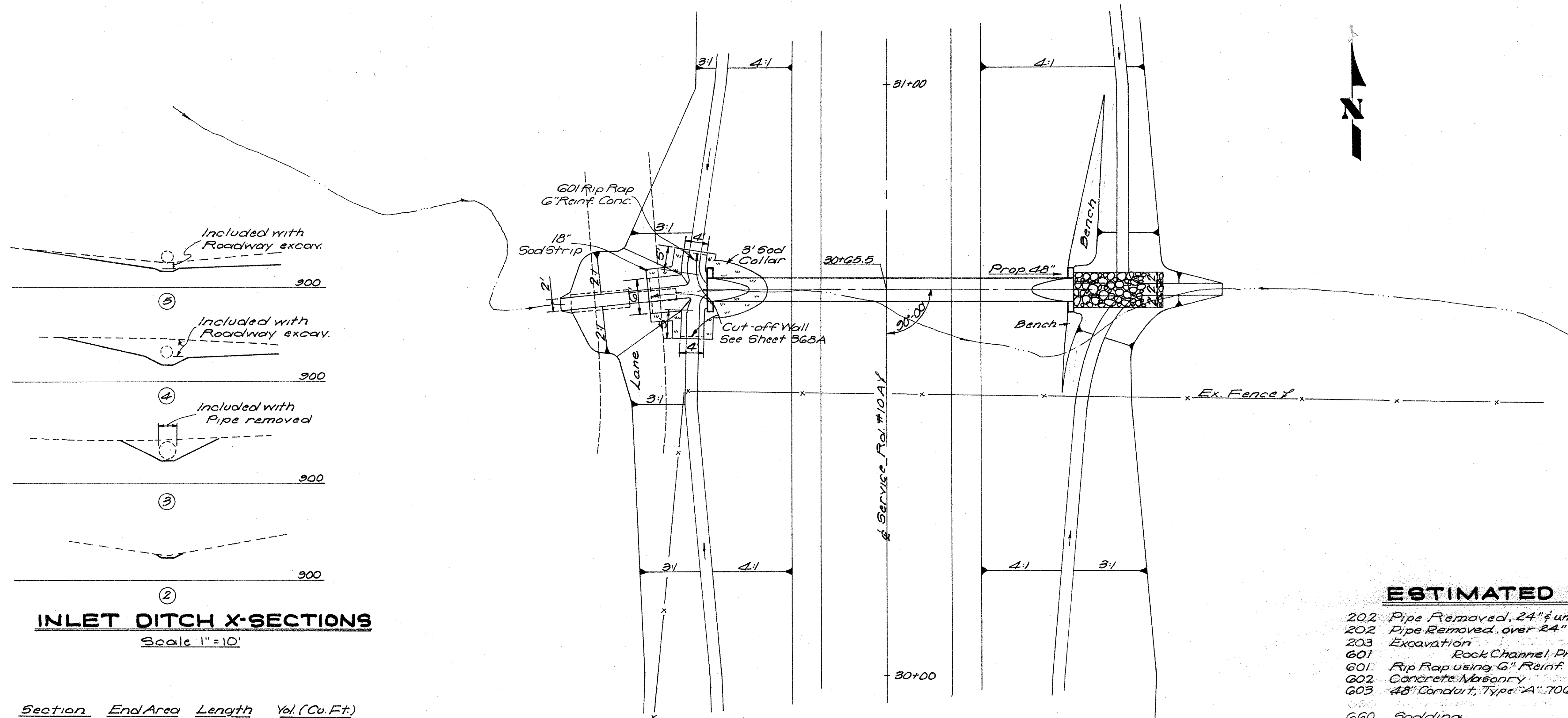
RAMP "D"		SERVICE RD. NO. 7		
203	Excavation	5 C.Y.	203 Excavation	2 C.Y.
601	Rip Rap using 6" Reinf. Conc.	124 S.Y.	601 Rip Rap using 6" Reinf. Conc.	52 C.Y.
602	Concrete Masonry	25 C.Y.	602 Protection Type B	25 C.Y.
603	66" Conduit Type "A" 706.03 Class V	213 L.F.	603 Concrete Masonry	240 L.F.
604	Catch Basin Mod. No. 2-G, as per Plan	1 Each	604 66" Conduit Type "A" 706.02 Class IV, 25' Radius	24 L.F.
600	Sodding	23 S.Y.		

DA	144 Ac	DA	169 Ac
Q80	244 cfs	Q25	227 cfs

PLAN
Scale 1" = 20'



SECTION ON 6" PIPE



INLET DITCH X-SECTIONS

Scale 1"=10'

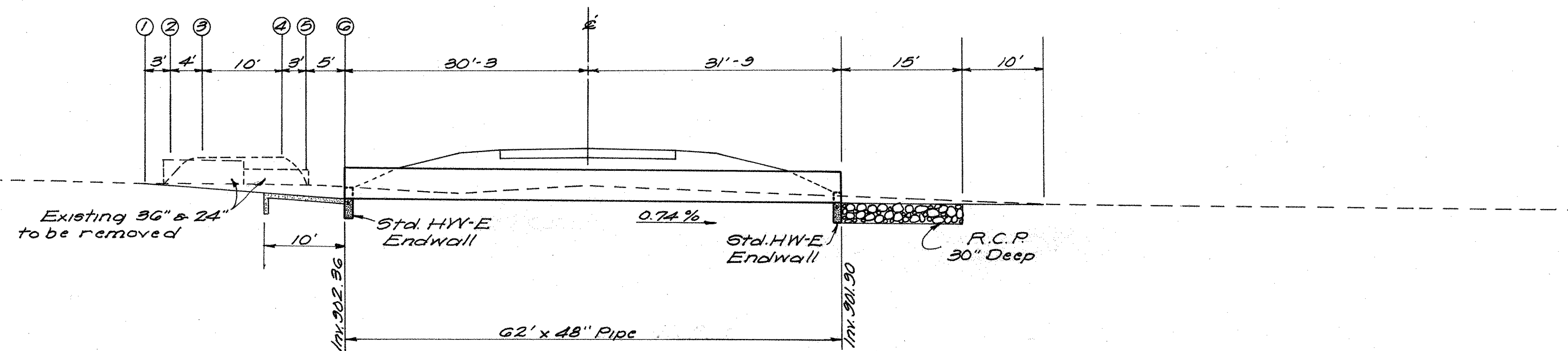
Section	End Area	Length	Vol. (Cu. Ft.)
①	0	3	3
②	2	4	48
③	22	10	145
④	7	3	14
⑤	2	5	5
⑥	0		
Sub-Total			215
Outlet ditch 3x10x1/2			15
Total			230
			9 Cu. Yds.

ESTIMATED QUANTITIES

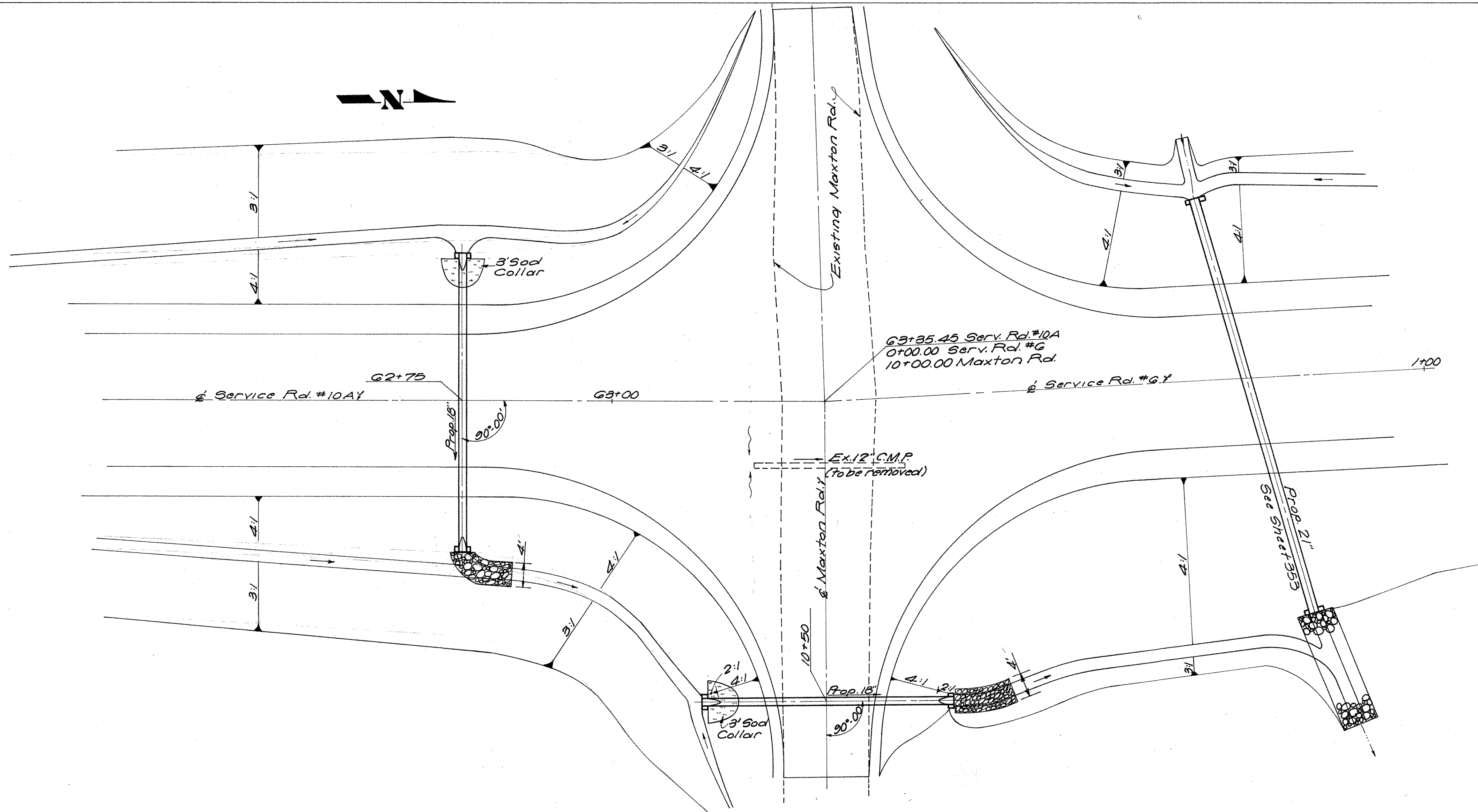
202	Pipe Removed, 24" & under	8 Lin. Ft.
202	Pipe Removed, over 24"	10 Lin. Ft.
203	Excavation	9 Cu. Yds.
601	Rock Channel Protection Type B	10 Cu. Yds.
601	Rip Rap using 6" Reinf. Conc.	11 Sq. Yds.
602	Concrete Masonry	17 Cu. Yds.
603	48" Conduit, Type "A" 706.02	62 Lin. Ft.
660	Sodding	9 Sq. Yds.

DA	66 Ac.
Q25	102 c.f.s.

PLAN
Scale 1"=10'



SECTION ALONG PIPE



PLAN
Scale 1"=10'

ESTIMATED QUANTITIES

SERVICE RD. NO. 10A

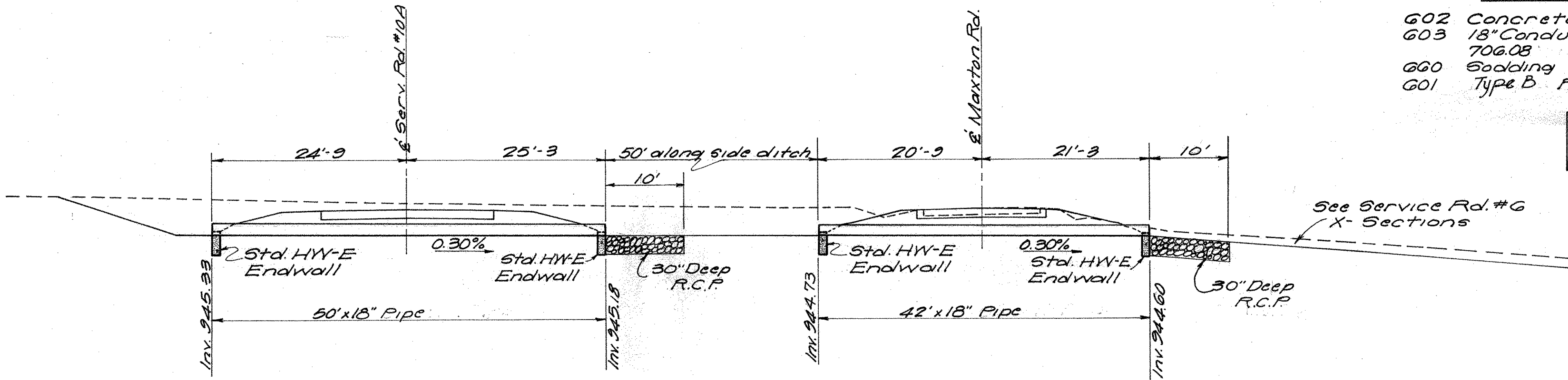
MAXTON RD.

602 Concrete Masonry	0.5 Cu. Yds.
603 18" Conduit, Type "A", 706.01, 706.02 or 706.03	50 Lin. Ft.
600 Sodding	3 Sq. Yds.
601 Type B Rock Channel Protection	4.4 Cu. Yds.

602 Concrete Masonry	0.5 Cu. Yds.
603 18" Conduit Type "A", 706.01, 706.02 or 706.03	42 Lin. Ft.
600 Sodding	3 Sq. Yds.
601 Type B Rock Channel Protection	4.4 Cu. Yds.

DA	1 Ac.
Q25	5 cfs.

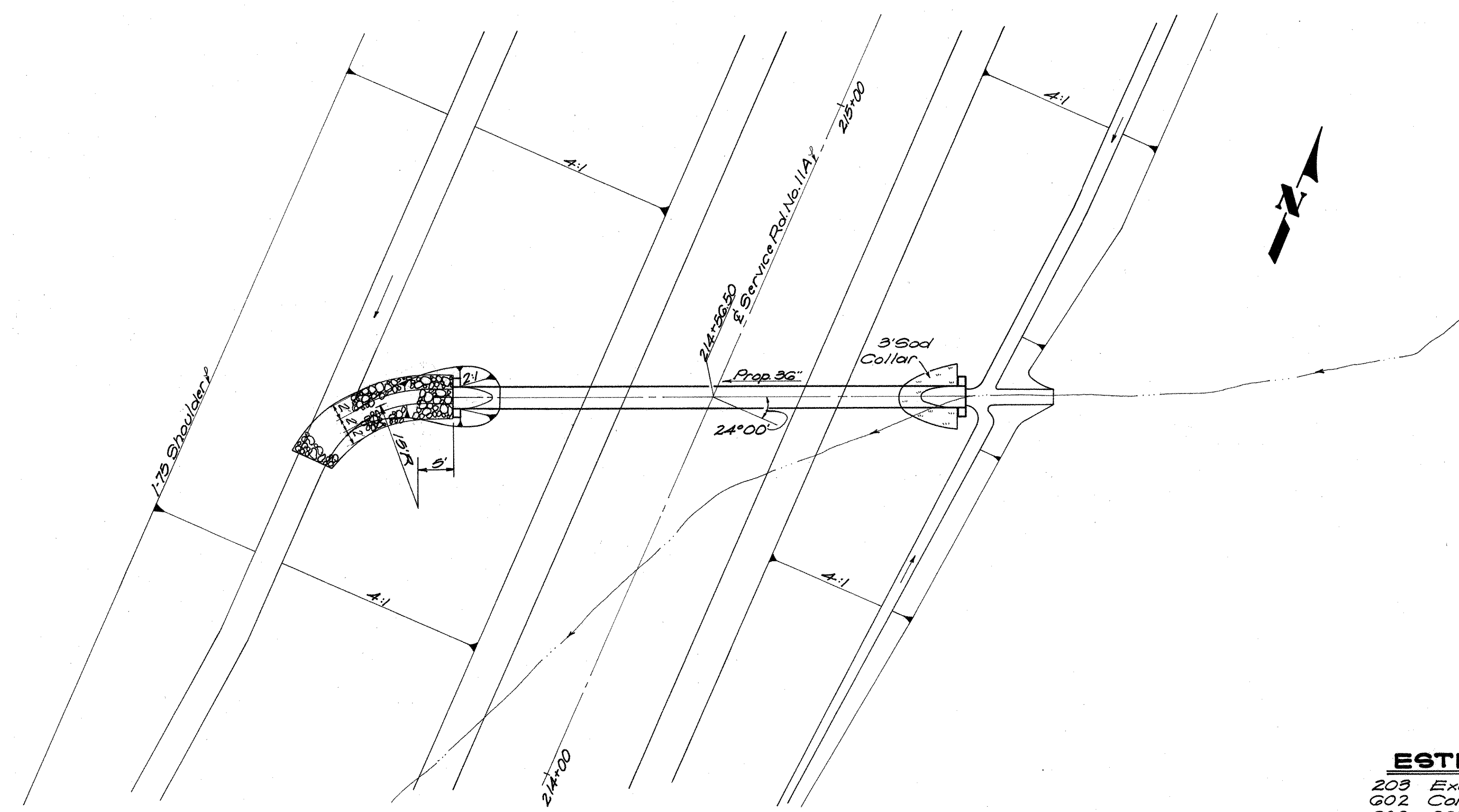
DA	2 Ac.
Q25	7 cfs.



SECTION ALONG & PIPE

PIPE CULVERT STA. 62+75 SERVICE RD. NO. 10A

PIPE CULVERT STA. 10+50 MAXTON RD.

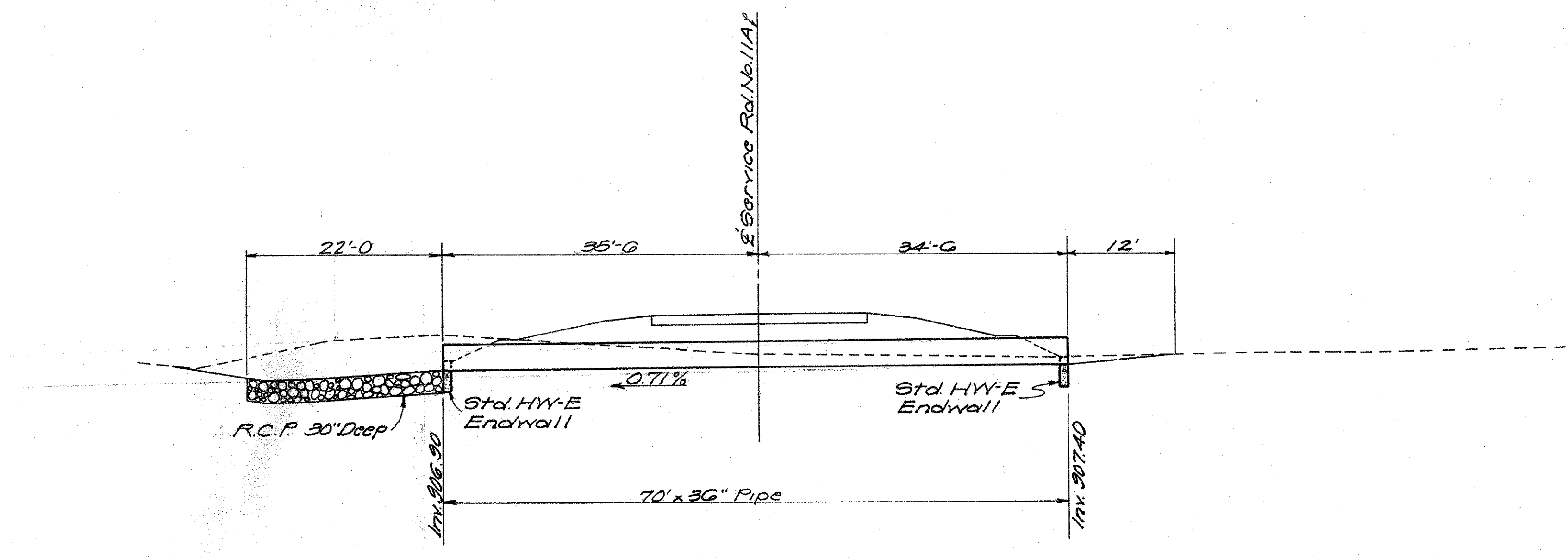


PLAN
 Scale 1"=10'

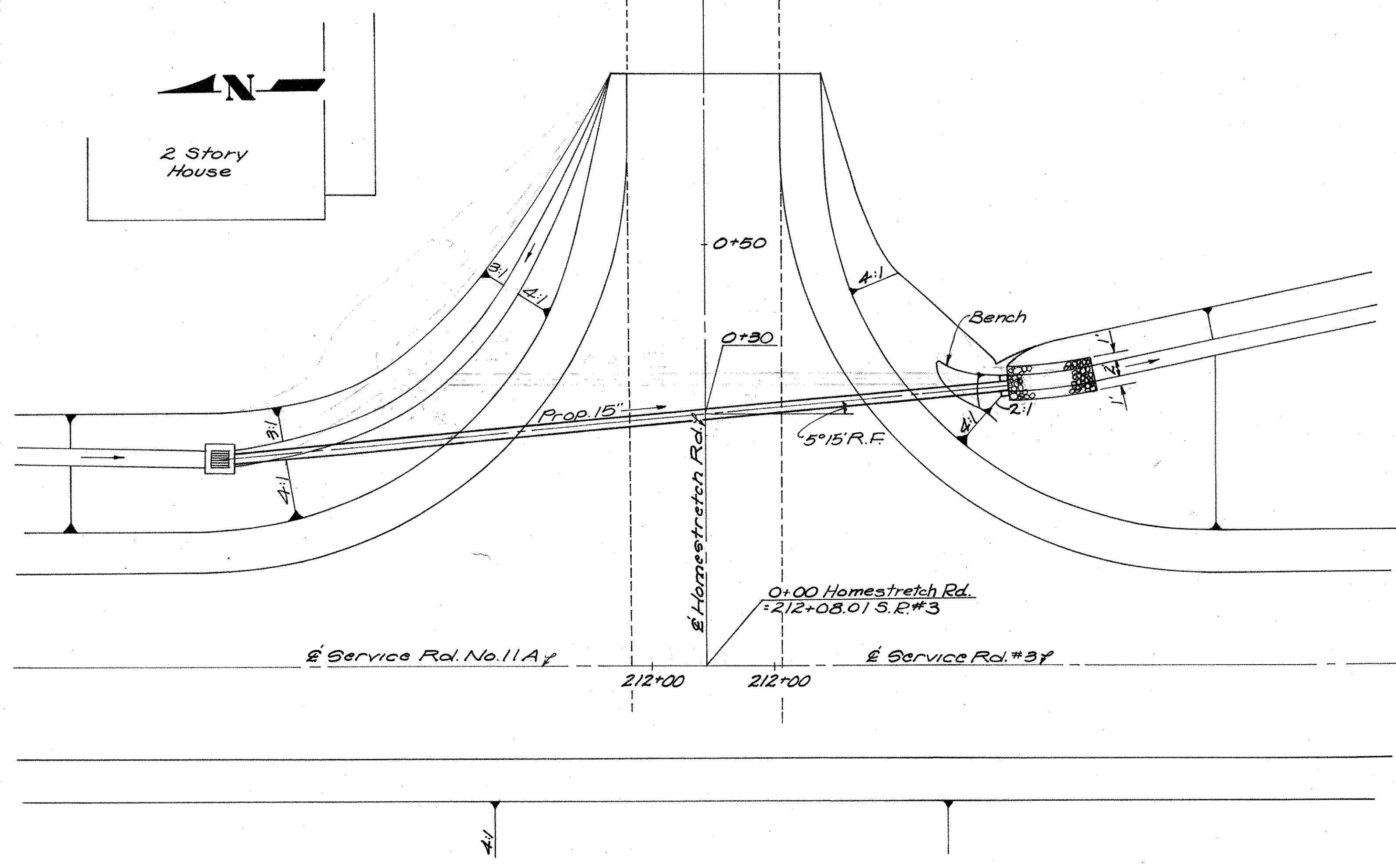
ESTIMATED QUANTITIES

203	Excavation	5 Cu. Yds.
602	Concrete Masonry	1.2 Cu. Yds.
603	36" Conduit, Type "A" 706.02 or 706.08	70 Lin. Ft.
660	Sodding	5 Sq. Yds.
601	Type B Rock Channel Protection	14.7 Cu. Yds.

DA	23 Ac.
Q25	59 C.F.S.



SECTION ALONG E PIPE

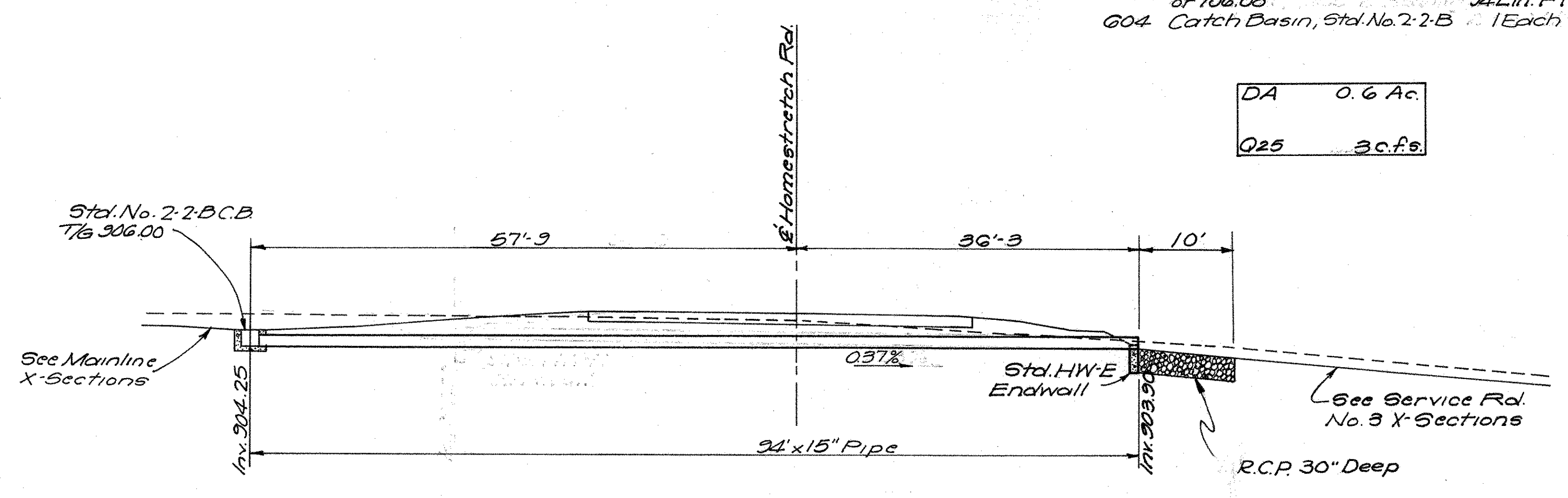


PLAN
Scale 1"=10'

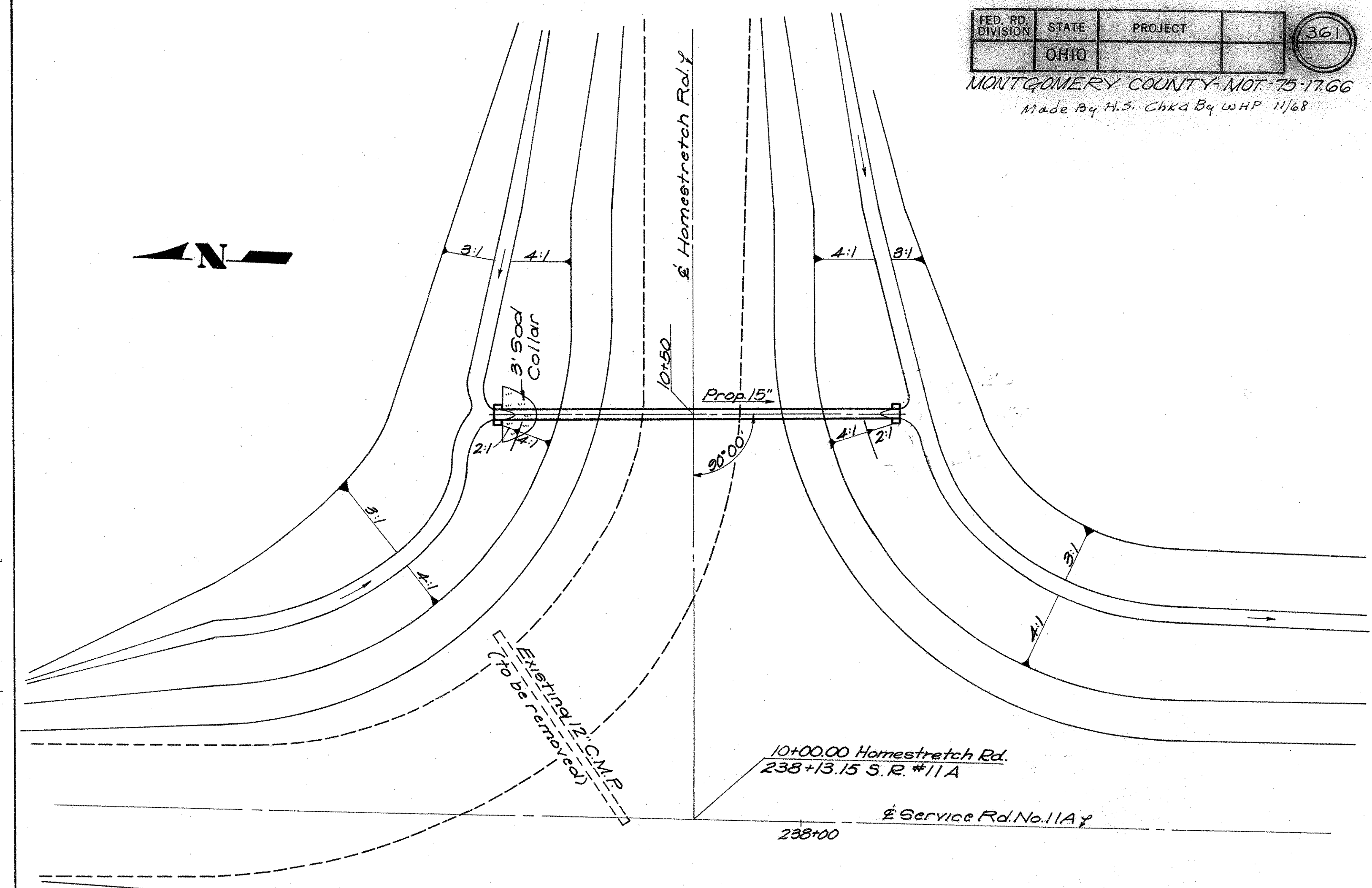
ESTIMATED QUANTITIES

- G01 Rock Channel Protection Type B 4.4 Cu.Yds.
- G03 15" Conduit, Type "B" 706.01, 706.02 or 706.03 24 Lin. Ft.
- G04 Catch Basin, Std. No. 2-2-B 1 Each

DA	0.6 Ac.
Q25	3 c.f.s.



SECTION ALONG & PIPE

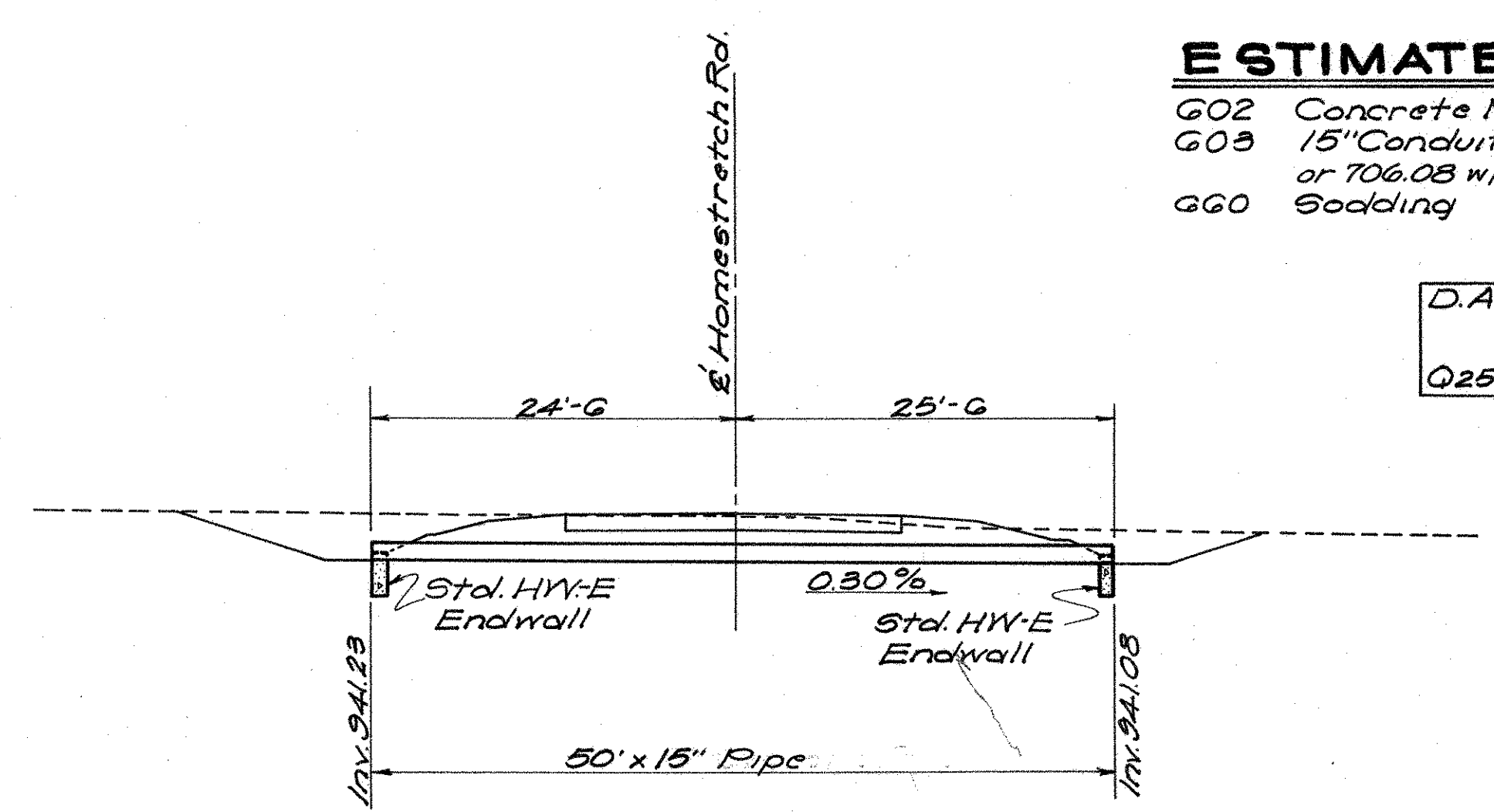


PLAN
Scale 1"=10'

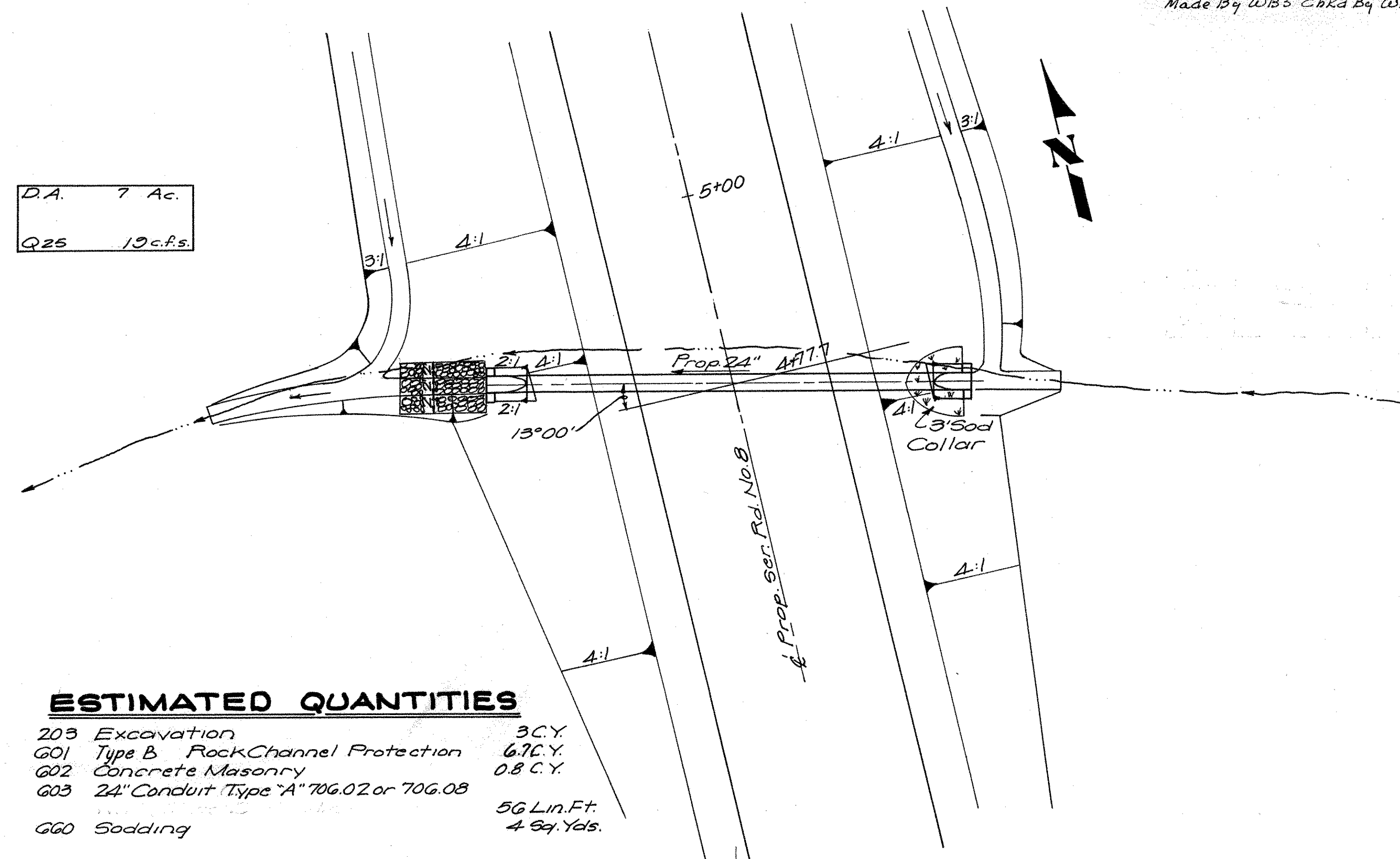
ESTIMATED QUANTITIES

- G02 Concrete Masonry 0.5 Cu.Yds.
- G03 15" Conduit, Type "A" 706.01, 706.02 or 706.03 w/Class "B" Bedding 50 Lin. Ft.
- G00 Sodding 2 Sq. Yds.

D.A.	0.5 Ac.
Q25	2 c.f.s.



SECTION ALONG & PIPE

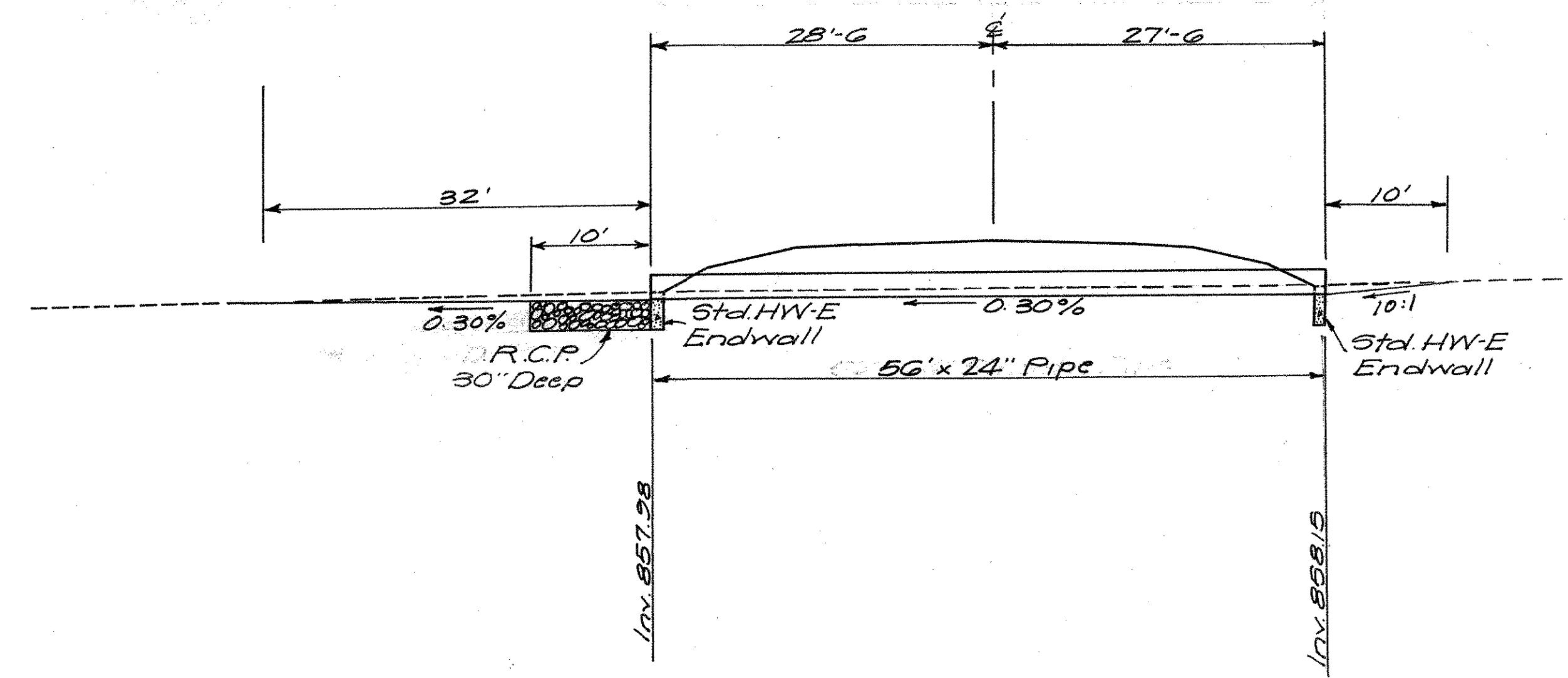


D.A. 7 Ac.
 Q25 19 c.f.s.

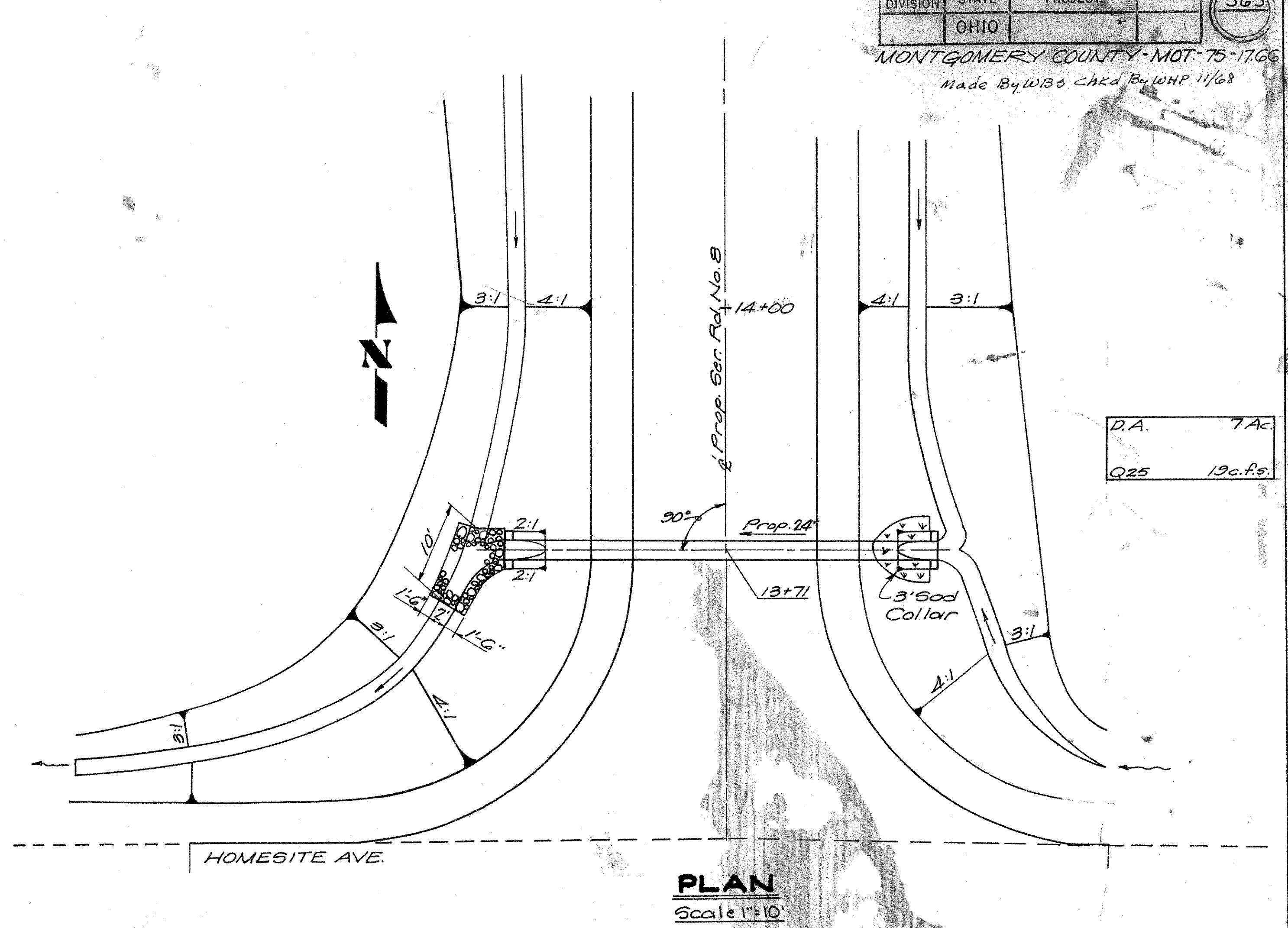
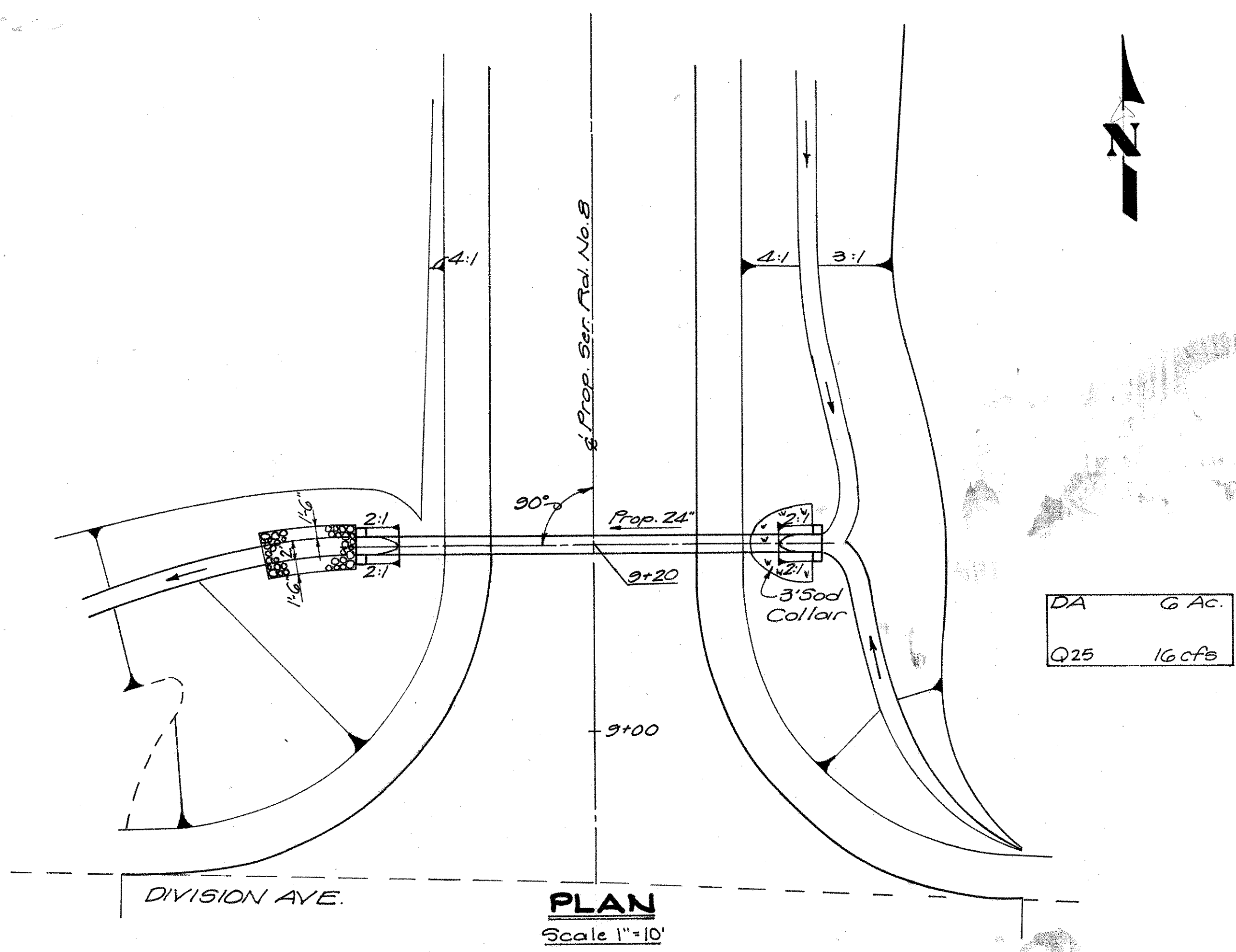
ESTIMATED QUANTITIES

203 Excavation	3 C.Y.
601 Type B Rock Channel Protection	67 C.Y.
602 Concrete Masonry	0.8 C.Y.
603 24" Conduit Type "A" 706.02 or 706.08	56 Lin. Ft.
660 Sodding	4 Sq. Yds.

PLAN
 Scale 1"=10'

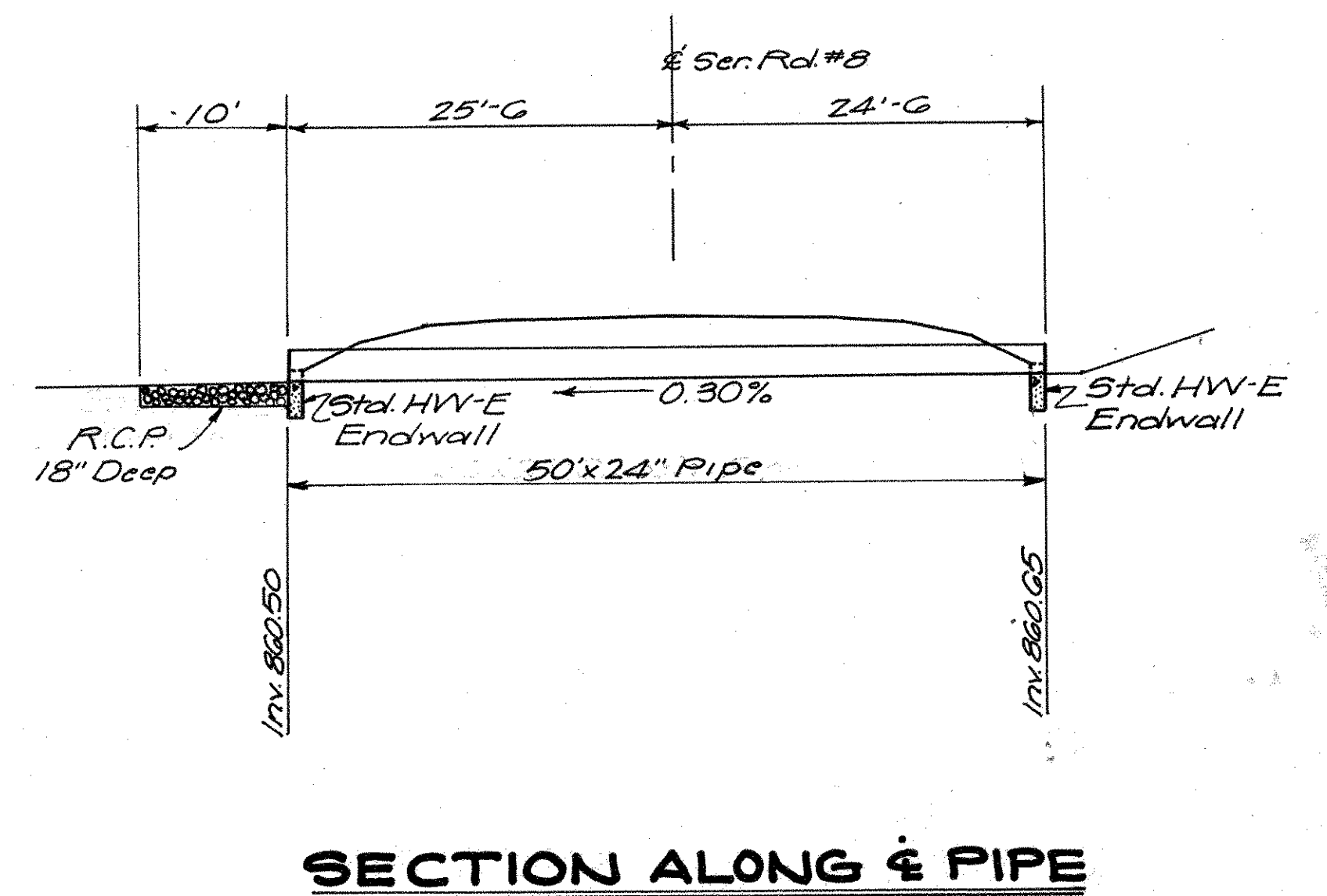


SECTION ALONG & PIPE

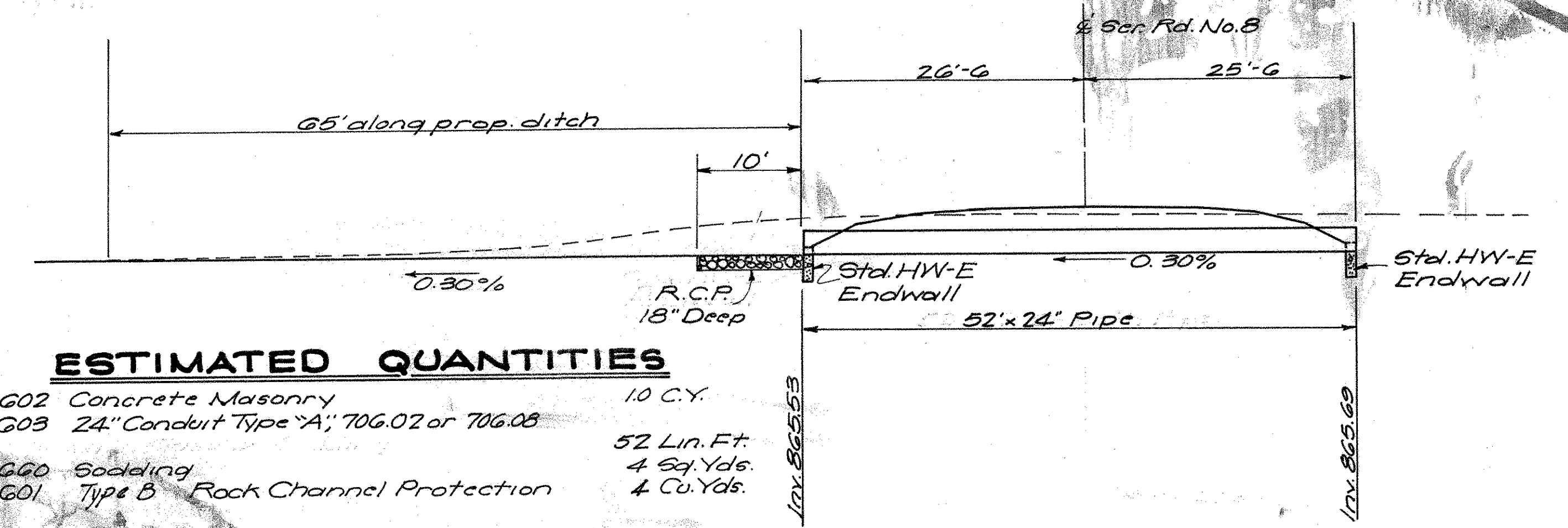


ESTIMATED QUANTITIES

G02 Concrete Masonry	0.8 C.Y.
G03 24" Conduit Type "A" 706.02 or 706.08	50 Lin. Ft.
G60 Sodding	4 Sq. Yds.
G01 Type B Rock Channel Protection	3.7 Cu. Yds.



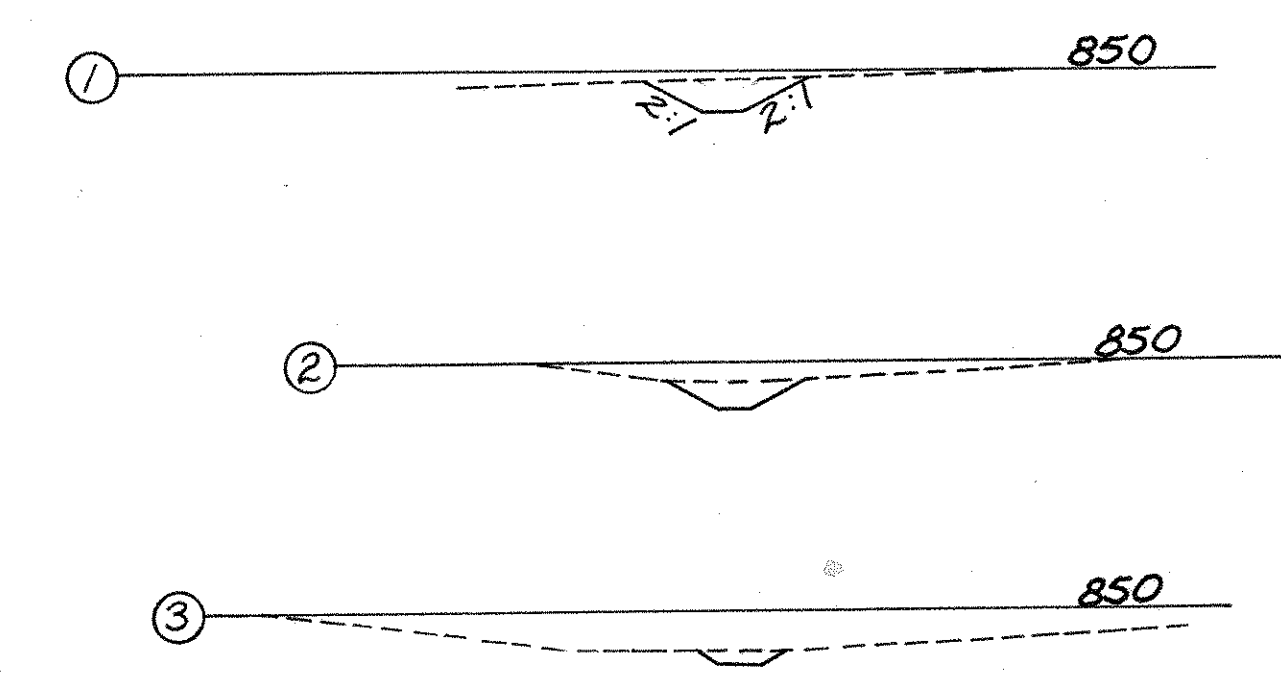
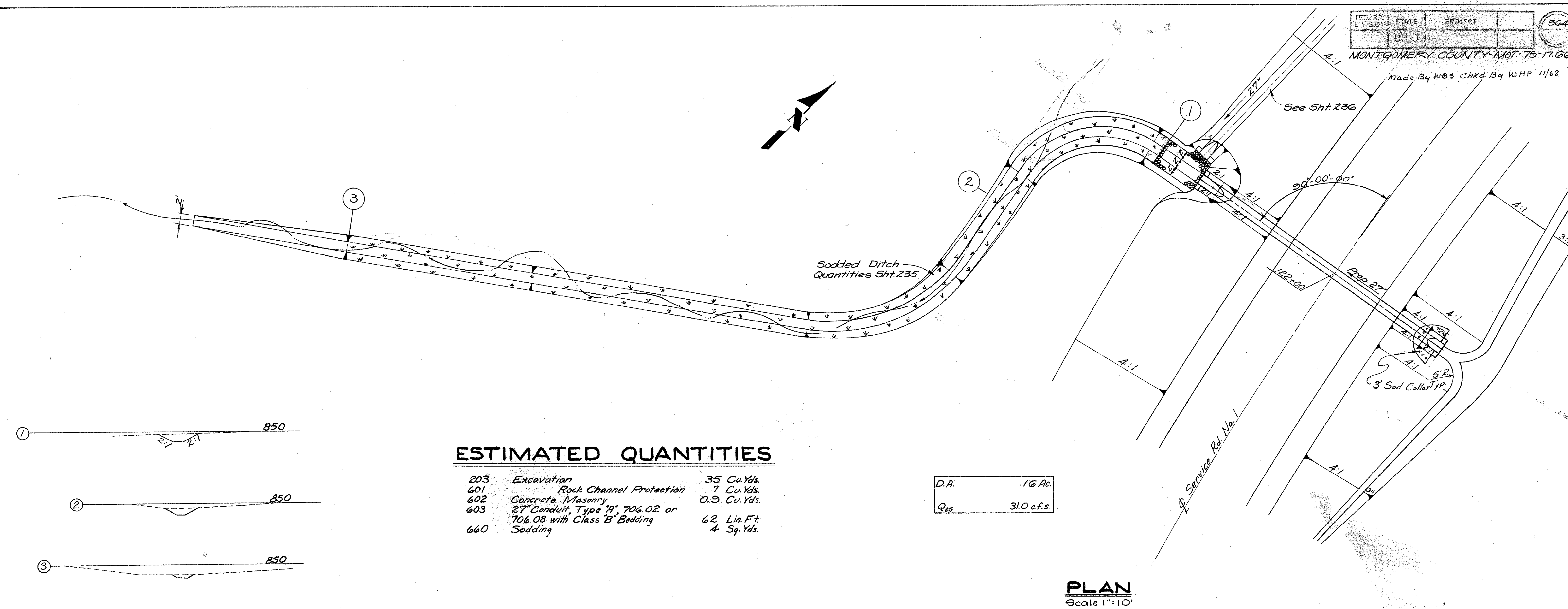
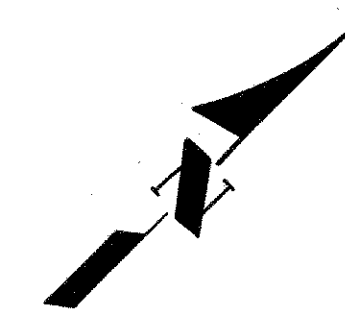
SECTION ALONG PIPE



ESTIMATED QUANTITIES

G02 Concrete Masonry	10 C.Y.
G03 24" Conduit Type "A" 706.02 or 706.08	52 Lin. Ft.
G60 Sodding	4 Sq. Yds.
G01 Type B Rock Channel Protection	4 Cu. Yds.

SECTION ALONG PIPE

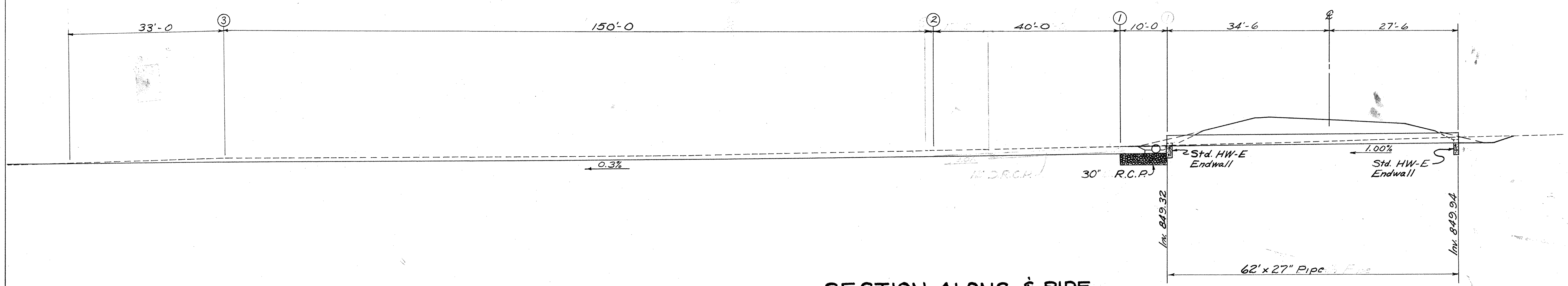


ESTIMATED QUANTITIES

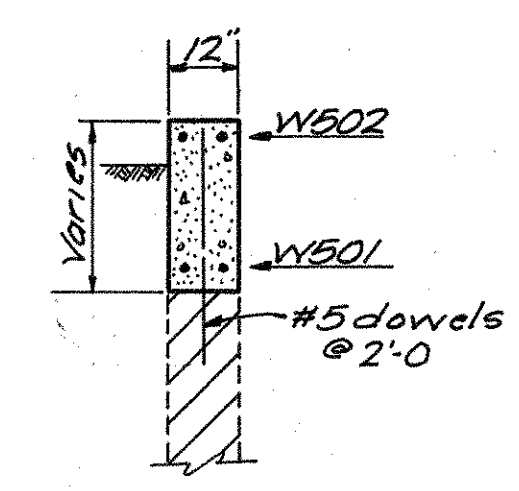
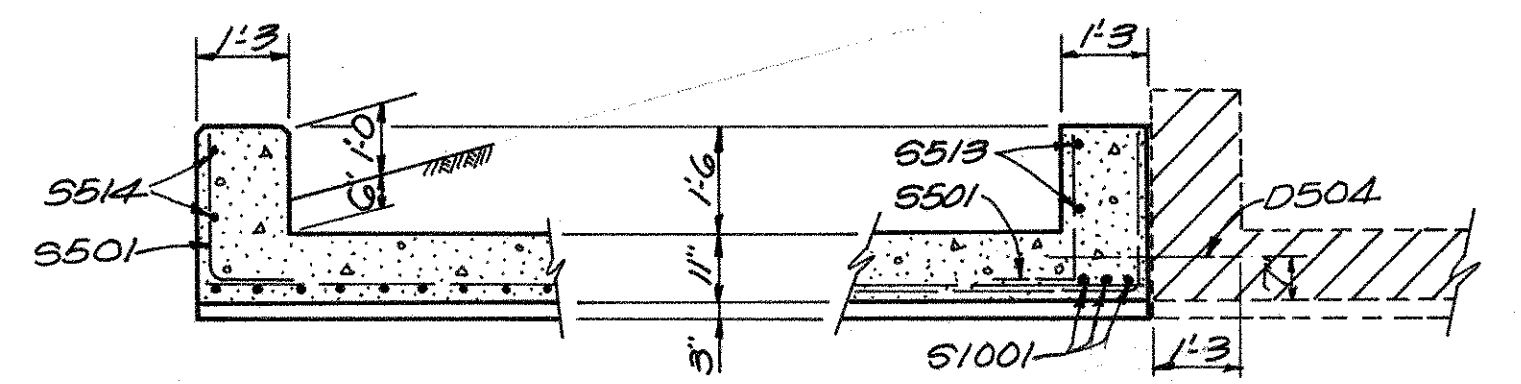
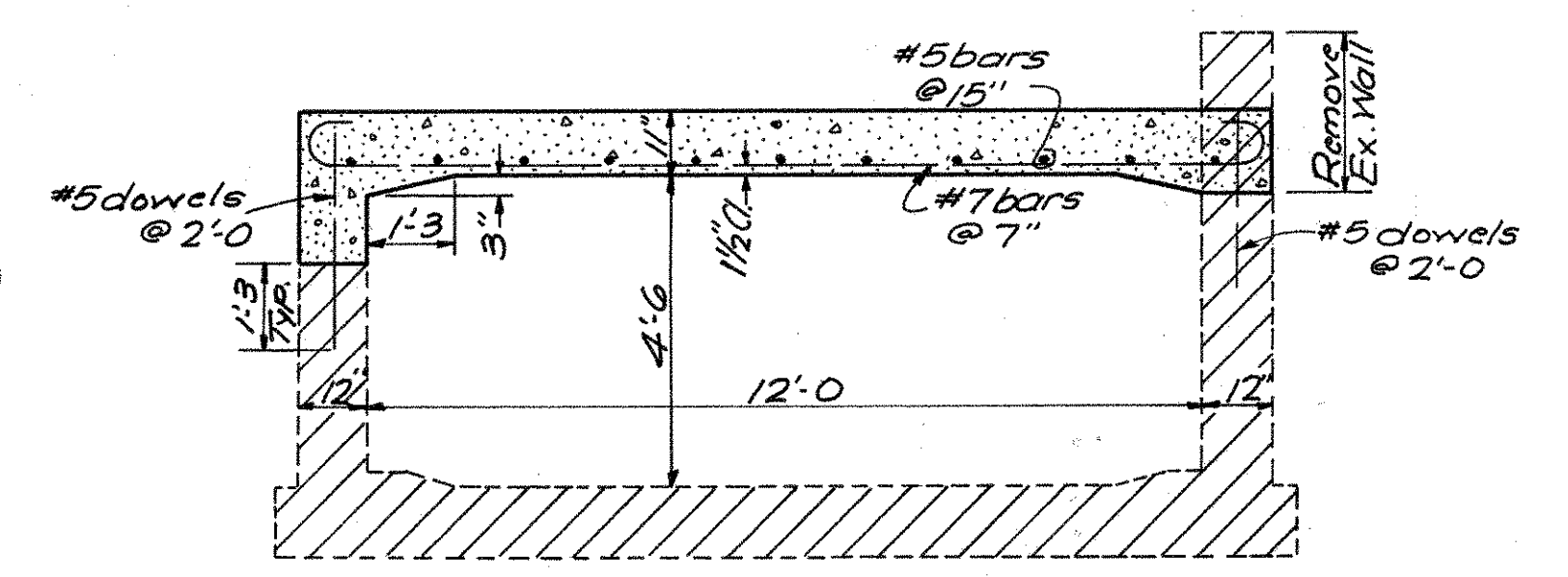
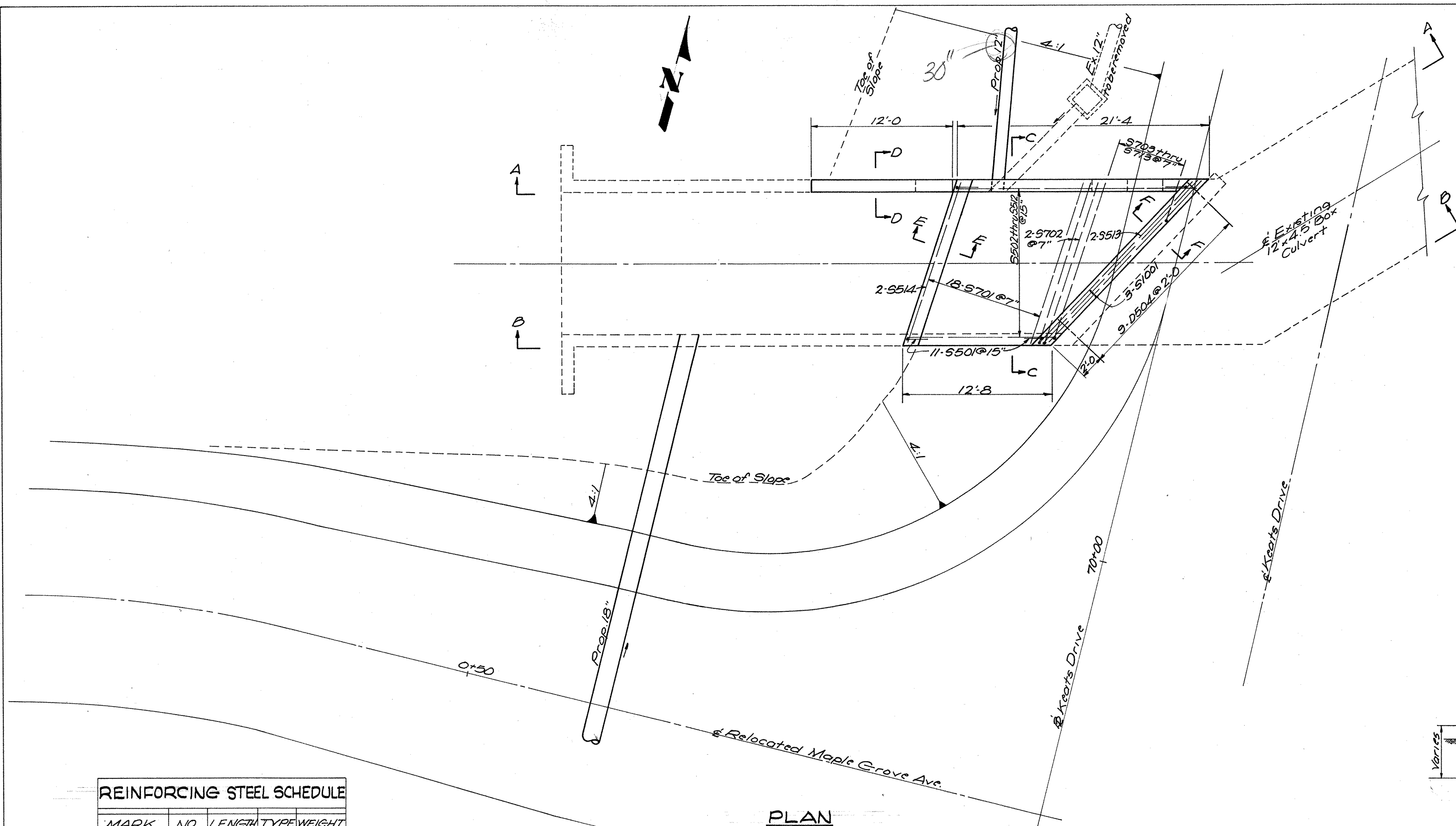
203	Excavation	35 Cu. Yds.
601	Rock Channel Protection	7 Cu. Yds.
602	Concrete Masonry	0.9 Cu. Yds.
603	27" Conduit, Type 'A', 706.02 or 706.08 with Class 'B' Bedding	62 Lin. Ft.
660	Sodding	4 Sq. Yds.

D.A.	116 Ac.
Q ₂₅	31.0 c.f.s.

PLAN
Scale 1"=10'



SECTION ALONG PIPE



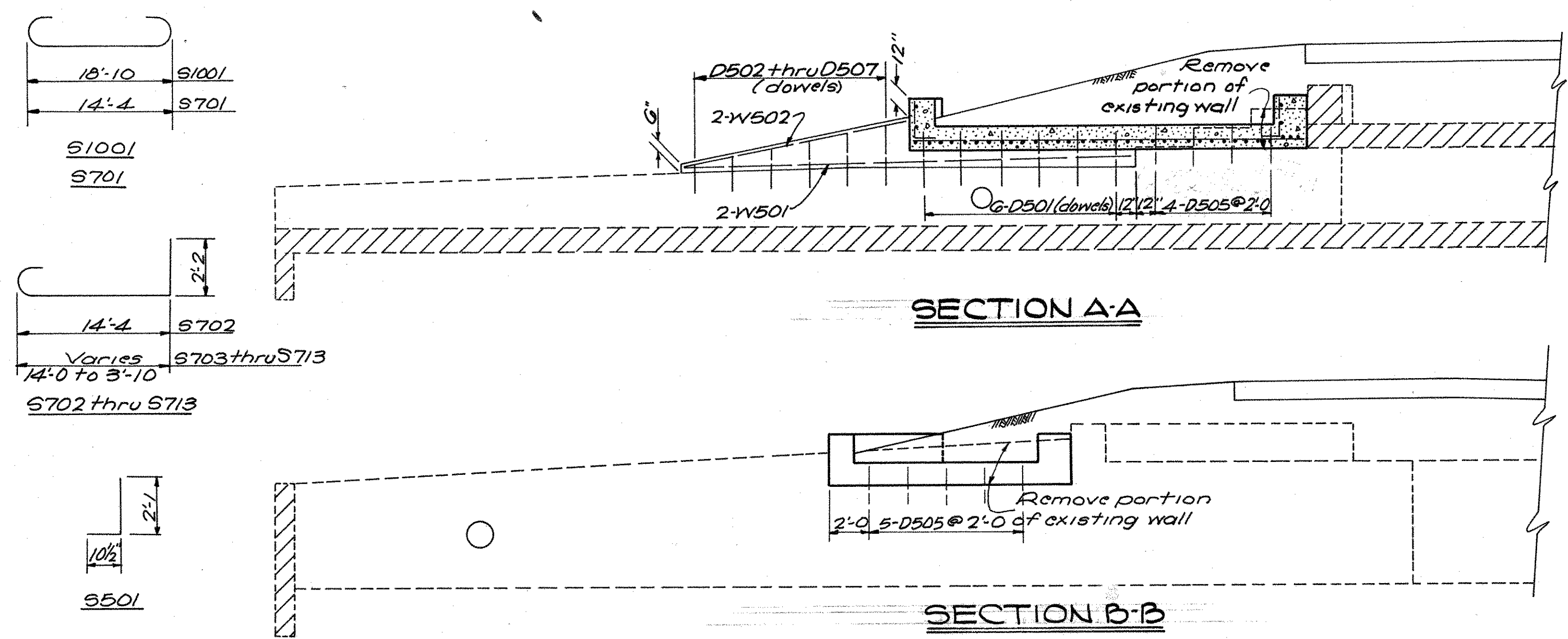
DA 507A.C.
 Q50 475 c.f.s.

REINFORCING STEEL SCHEDULE

MARK	NO.	LENGTH	TYPE	WEIGHT
S1001	3	22'-1"	Bt.	285
S701	18	16'-0"	Bt.	539
S702	2	17'-2"	Bt.	70
S703 thru S713	11	Varies*	Bt.	264
S501	22	2'-10"	Bt.	65
S502 thru S512	11	Varies*	St.	130
S513	2	18'-10"	St.	33
S514	2	14'-4"	St.	30
W501	2	23'-4"	St.	49
W502	2	11'-8"	St.	24
D501	6	3'-1"	St.	19
D502	1	3'-5"		4
D503	1	3'-0"		3
D504	10	2'-8"		28
D505	10	2'-3"		23
D506	1	2'-0"		2
D507	1	1'-8"	St.	2
R701	1	9'-6"	St.	**
R501	1	6'-7"	St.	**
Total Weight				1686

* Length Varies 16'-10" to 6'-8"
 * Length Varies 20'-6" to 12'-8"
 ** Replacement Bars

PLAN



SECTION D-D

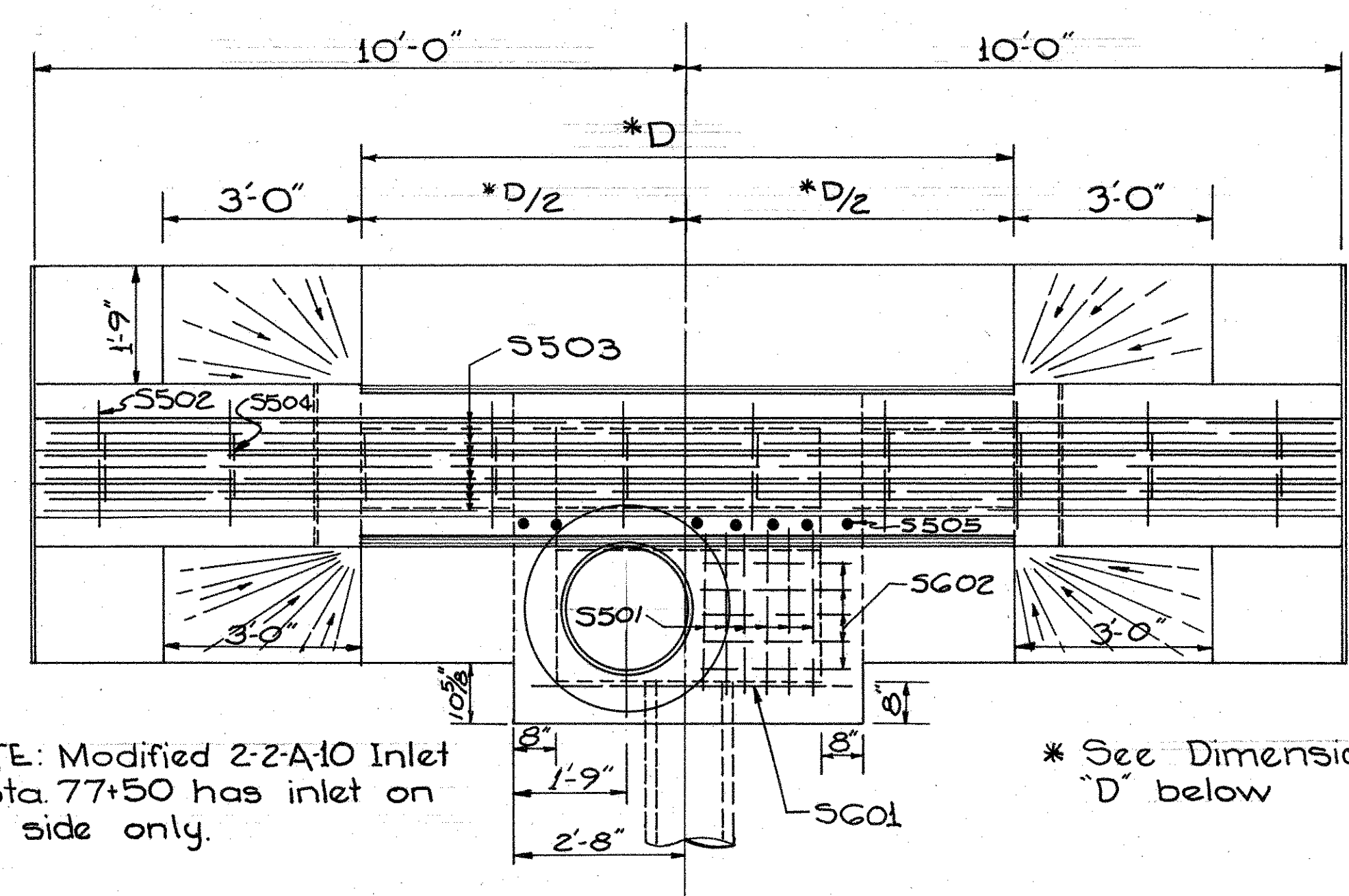
ESTIMATED QUANTITIES

Quantity	Description	Unit
202	Portions of existing structures removed	Lump Sum
509	Reinforcing steel	1686 Lbs.
510	Dowel holes	38 Lin. Ft.
511	Class "C" concrete	12 Cu. Yds.

ESTIMATED QUANTITIES SUB-SUMMARY

ITEM	SHEET NUMBER																														ITEM	TOTAL TO GENERAL SUMMARY	UNIT	DESCRIPTION					
	330	331	331A	332	333	334	335	336	337	338	339	340	341	342	344	345	347	348	349	350	351	352	353	354	355	356	358	359	360	361					362	363	364	365	
202																																			L.S.	202	Lump Sum	Lump Sum	Portions of existing structures removed
202										L.S.						L.S.																				202	Lump Sum	Lump Sum	Existing structures removed
202						8			8		8	8																								202	42	L.F.	Pipe removed over 24"
202						8	114		8		8		107																							202	319	L.F.	Pipe removed 24" and under
202																																				202	3	Each	Catch basins removed
202																																				202	5	Each	Inlets Removed
203																																				203	134	C.Y.	Excavation
203																																				203	52	C.Y.	Embankment
601																																				601	332	S.Y.	Riprap using 6" reinforced concrete slab
601																																				601	368.6	C.Y.	Rock channel protection, Type B
601																																				601	143.1	C.Y.	Rock channel protection, Type A
602																																				602	60.1	C.Y.	Concrete Masonry
603																																				603	453	L.F.	66" Conduit, Type "A" 706.03 Class V with Class "B" bedding
603																																				603	24	L.F.	66" Conduit, Type "A" 706.02 Class IV, 25' Radius with Class "B" bedding
603																																				603	72	L.F.	66" Conduit, Type "A" 706.02 with Class "B" bedding
603																																				603	20	L.F.	66" Conduit, Type "A" 706.02, 50' Radius with Class "B" bedding
603																																				603	122	L.F.	66" Conduit, Type "A" 706.02 Class III with Class "B" bedding
603																																				603	110	L.F.	60" Conduit, Type "A" 706.02 with Class "B" bedding
603																																				603	62	L.F.	48" Conduit, Type "A" 706.02 with Class "B" bedding
603																																				603	190	L.F.	29"x45" Conduit, Type "A" 706.04 Class HE.II with Class "B" bedding
603																																				603	136	L.F.	48" Conduit, Type "A" 706.02 Class IV with Class "B" bedding
603																																				603	144	L.F.	36" Conduit, Type "A" 706.02 or 706.08 Class III with Class "B" bedding
603																																				603	24	L.F.	36" Conduit, Type "A" 706.02 or 706.08, 25' Radius with Class "B" bedding
603																																				603	175	L.F.	24"x38" Conduit, Type "A" 706.04 Class HE.II with Class "B" bedding
603																																				603	708	L.F.	36" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	178	L.F.	33" Conduit, Type "A" 706.02 or 706.08 Class III with Class "B" bedding
603																																				603	160	L.F.	33" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	168	L.F.	30" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	120	L.F.	30" Conduit, Type "A" 707.05 with Class "B" bedding
603																																				603	136	L.F.	30" Conduit, Type "A" 706.02 Class II with Class "B" bedding
603																																				603	62	L.F.	27" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	583	L.F.	24" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	106	L.F.	24" Conduit, Type "A" 706.02 or 706.08 Class III with Class "B" bedding
603																																				603	188	L.F.	21" Conduit, Type "A" 706.02 Class II with Class "B" bedding
603																																				603	182	L.F.	21" Conduit, Type "A" 706.02 or 706.08 with Class "B" bedding
603																																				603	298	L.F.	18" Conduit, Type "A" 706.01, 706.02 or 706.08 with Class "B" bedding
603																																				603	116	L.F.	18" Conduit, Type "A" 706.02 Class II with Class "B" bedding
603																																				603	50	L.F.	15" Conduit, Type "A" 706.01, 706.02 or 706.08 with Class "B" bedding
603																																				603	38	L.F.	30" Conduit, Type "D"
510																																				510	38	L.F.	Dowel Holes
603																																				603	120	L.F.	24" Conduit, Type "B" 706.02 Class III or 706.08
603																																				603	67	L.F.	21" Conduit, Type "B"
603																																				603	67	L.F.	18" Conduit, Type "B"
603																																				603	98	L.F.	15" Conduit, Type "B"
503																																				503	589	C.Y.	Unclassified Excavation
509																																				509	38262	Lbs.	Reinforcing Steel
511																																				511	280	C.Y.	Class "C" Concrete
604																																				604	1	Each	Catch Basins, Std. No. 2-2-B
604																																				604	2	Each	Catch Basins, Mod. No. 5 as per plan
604																																				604	1	Each	Catch Basins, Std. No. 2-4
604																																				604	1	Each	Catch Basins, Mod. No. 2-G as per plan
604																																				604	5	Each	Catch Basins, Std. No. 5
604																																				604	4	Each	Catch Basins, Mod. No. 2-4 as per plan
604																																				604	1	Each	Inlets, Mod. No. 2-A-G as per plan
604																																				604	1	Each	Inlets, Mod. No. 2-A-10 as per plan
604																																				604	2	Each	Inlets, Std. No. 2-A-12
604																																				604	1	Each	Inlets, Mod. No. 2-A-8 as per plan
600																																				600	257	S.Y.	Sodding

MOJ-75-17.66



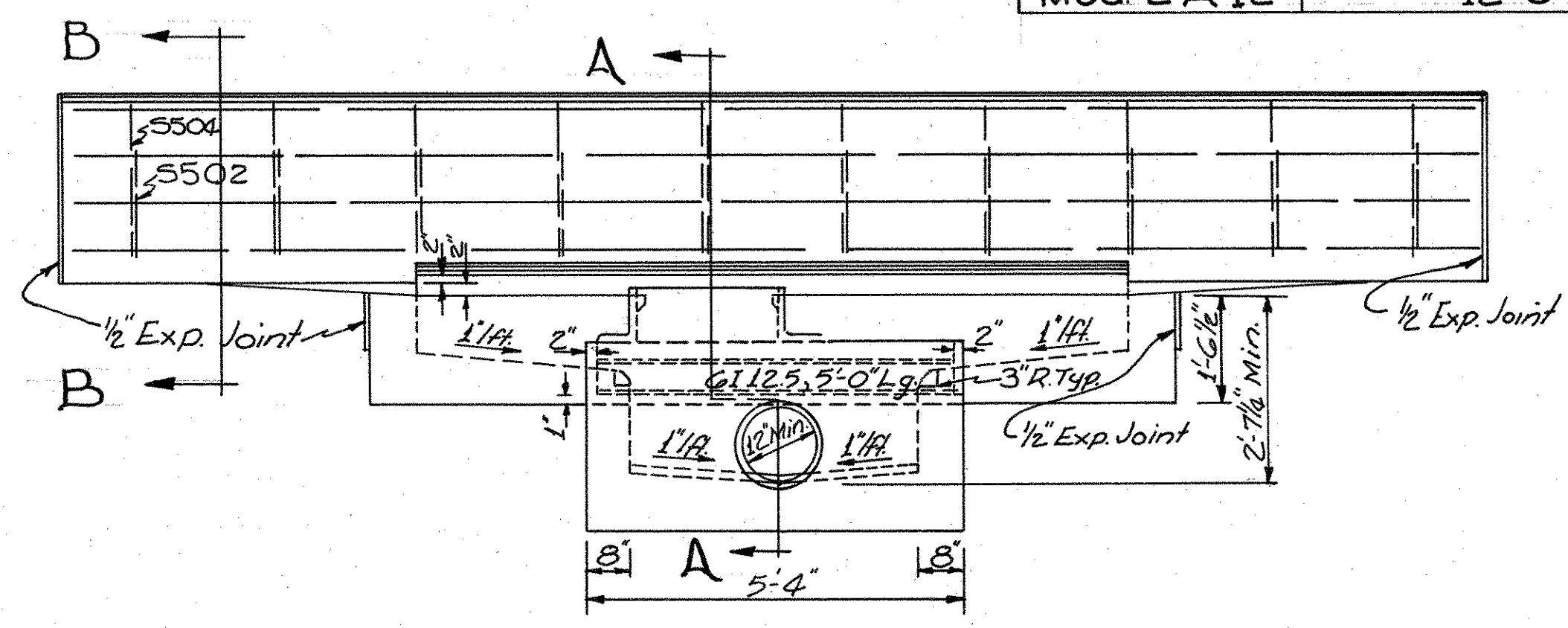
NOTE: Modified 2-A-10 Inlet @ Sta 77+50 has inlet on one side only.

* See Dimension "D" below

PLAN

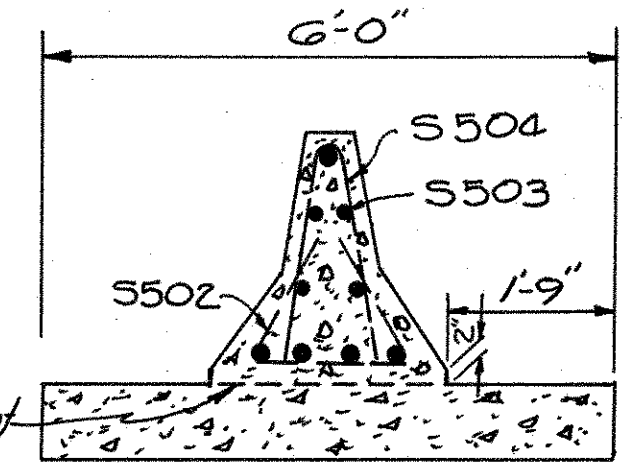
TABLE FOR DIMENSION "D"

INLET	DIMENSION "D"
Mod. 2-A-6	6'-0"
Mod. 2-A-8	8'-0"
Mod. 2-A-10	10'-0"
Mod. 2-A-12	12'-0"

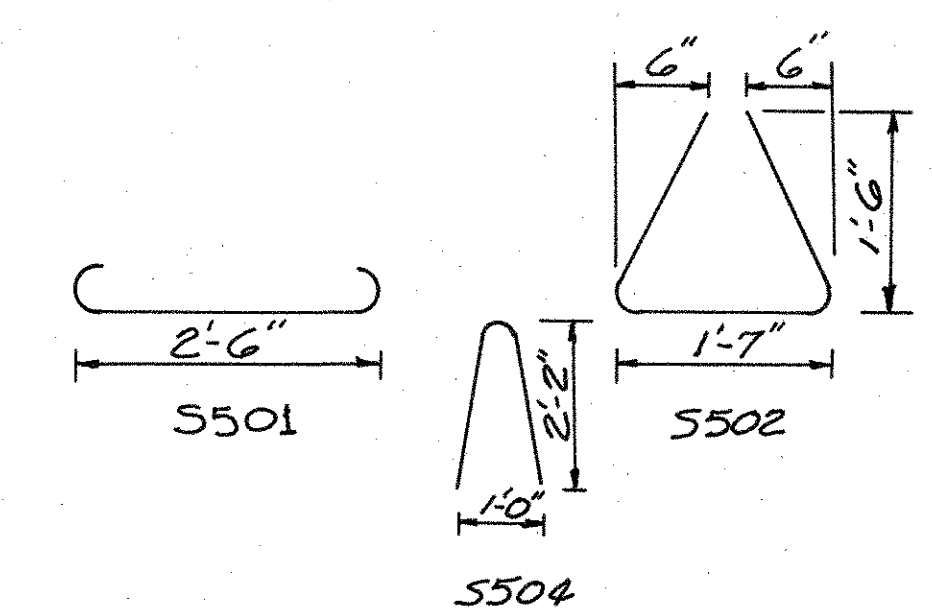


ELEVATION

Optional Construction Joint See Sheet 19 B



SECTION "B-B"

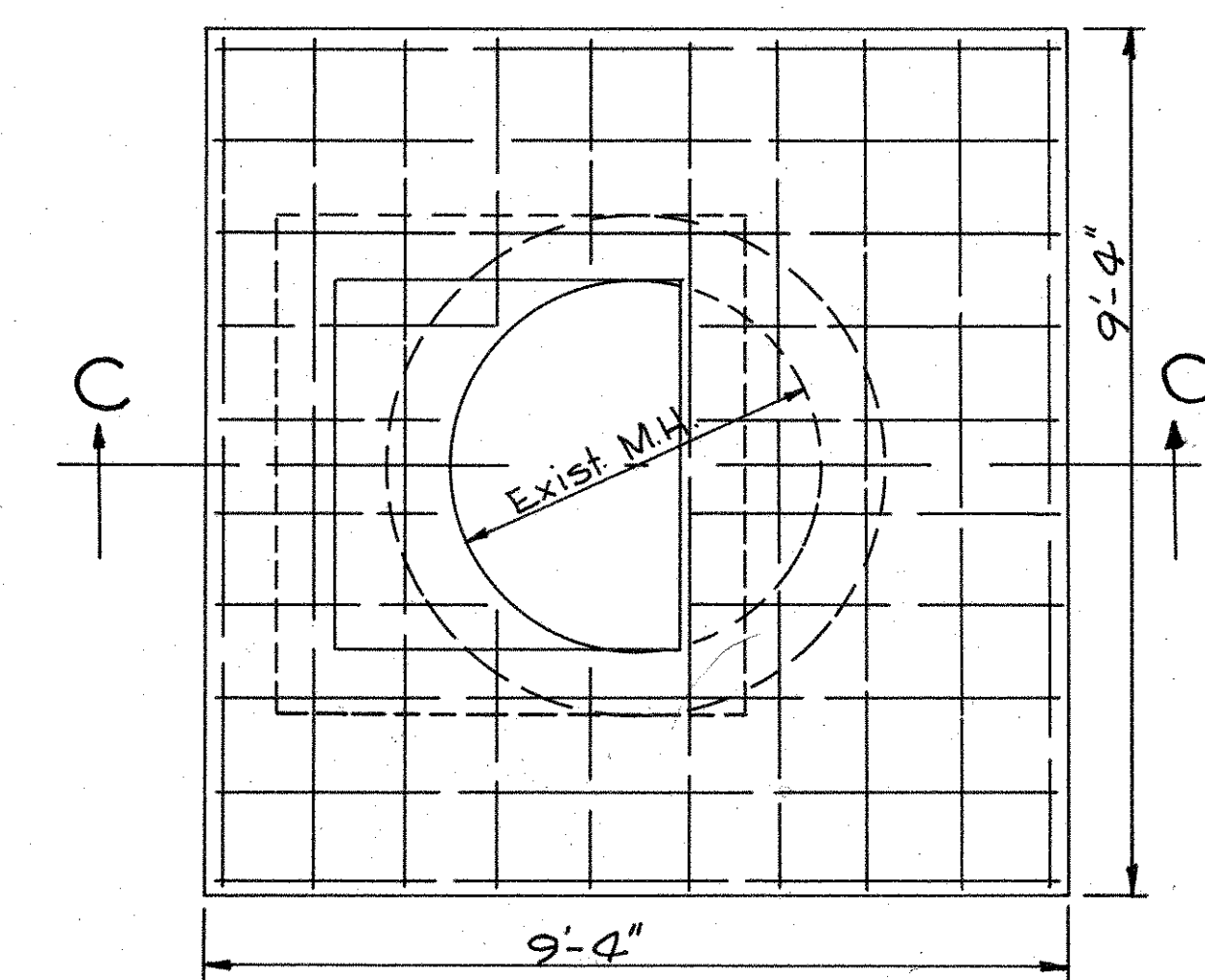


REINFORCING STEEL

Mark	Shape	Length	N ^o	Weight
S501	Bent	3'-8"	6	23
S502	Bent	4'-11"	10	51
S503	Str.	19'-8"	9	184
S504	Bent	4'-2"	10	43
S505	Str.	1'-2"	7	9
S506	Str.	6'-2"	1	10
S507	Str.	2'-4"	5	18

Total 338 lbs.

NOTE: Payment for furnishing & placing reinforcing steel shall be included in the unit price bid for Concrete Median Barrier Type A.

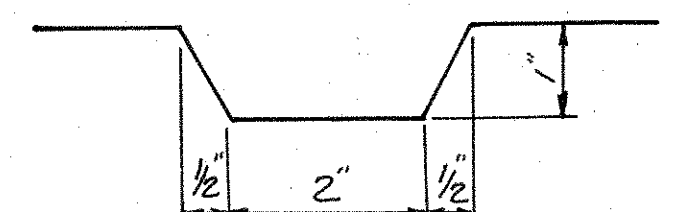


PLAN

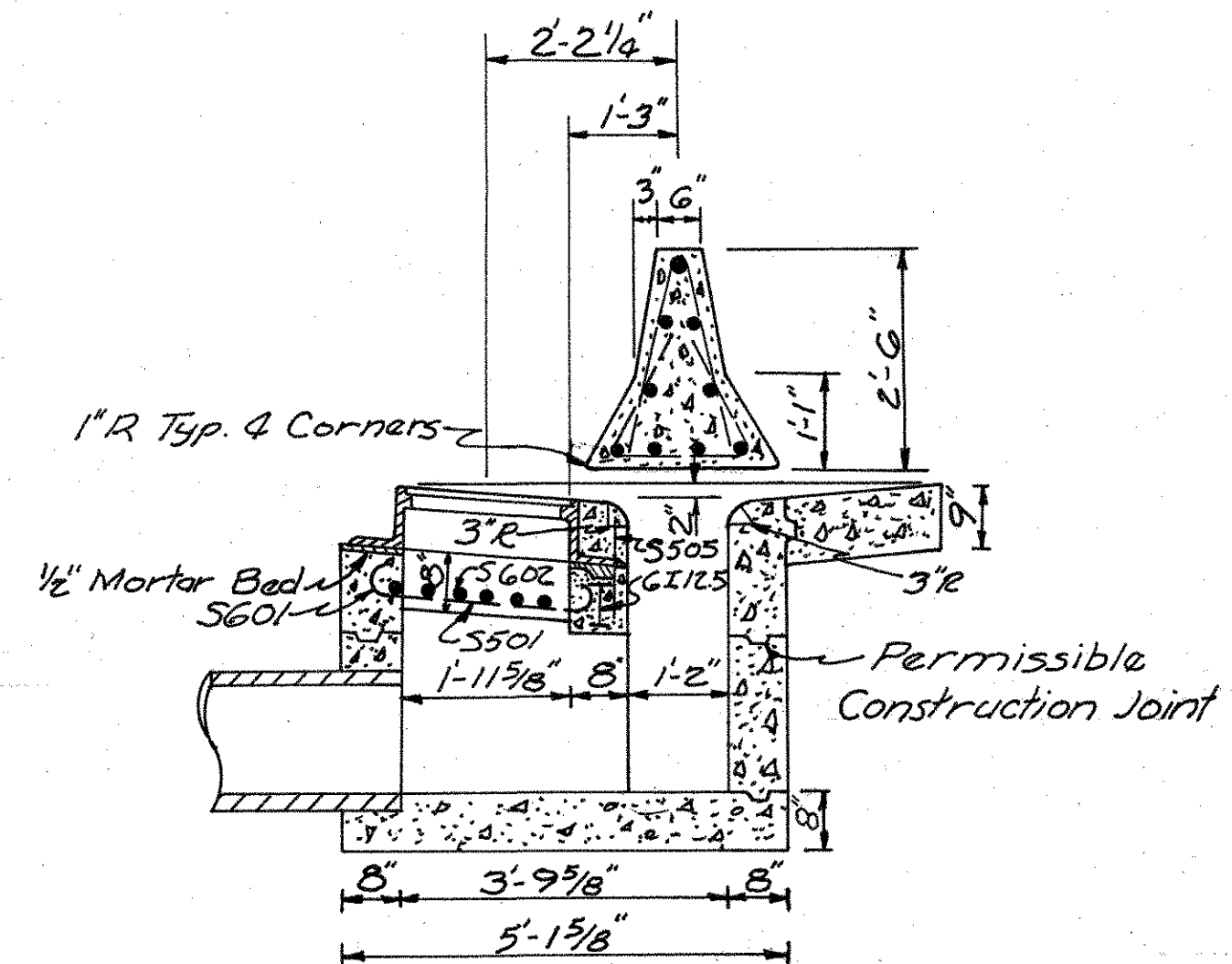
NOTE: Use 1/2 inch diameter bars each way. Payment for furnishing & placing reinforcing steel shall be included in unit price bid for Modified Inlets.

NOTES: Portion above inlet to be paid for as Concrete Median Barrier Type A. Furnish and install steps 12 inches in inlets which are deeper than 4 feet. For detail of Manhole Casting see Std. Drawing M.H.-1 and for notes not shown see Std. Drawing I-2A.

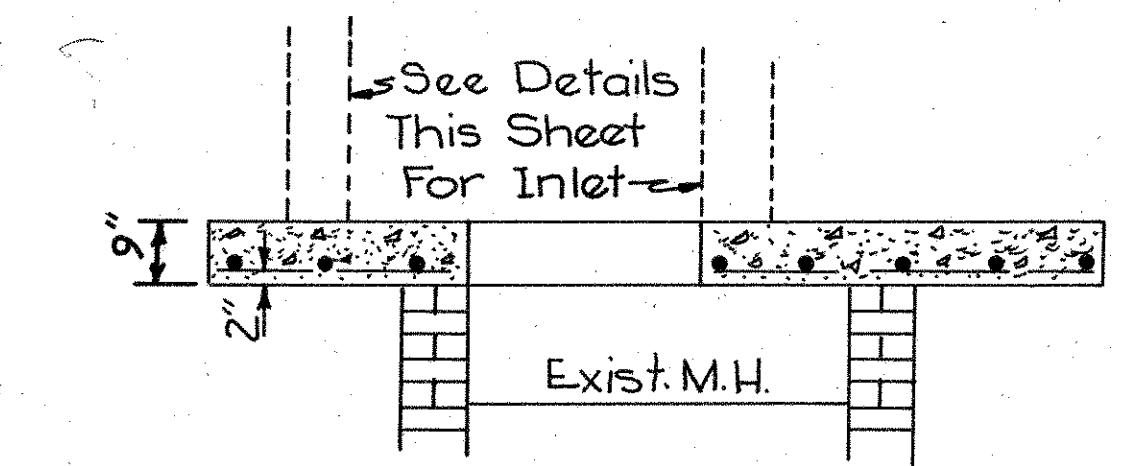
WALLS: The sections between the base and the upper permissible construction joint may be built of brick, concrete block or cast-in-place concrete, if basin is less than 12 feet deep. The unit above the upper permissible construction joint may be precast as an alternate to cast-in-place. Concrete for precast or cast-in-place construction shall meet the requirements of Section 511 Class C of the material specifications. If a skewed pipe protrudes more than 2 inches inside a wall, the pipe shall be trimmed flush and finished to produce a neat appearance. If basin is deeper than 12 feet, it shall be built of class C concrete reinforced by placing 1/2 inch diameter bars 12 inches both vertically and horizontally, with a 2 inch clearance from inside face of wall.



KEY JOINT

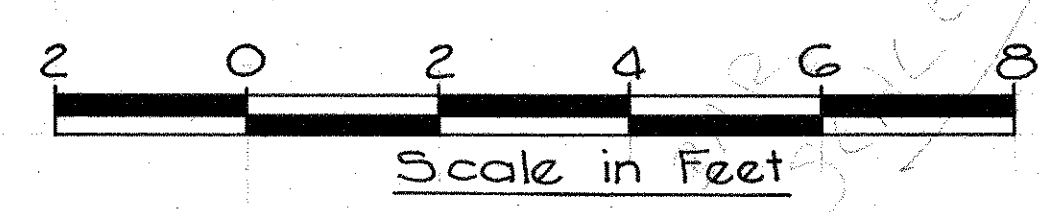


SECTION "A-A"



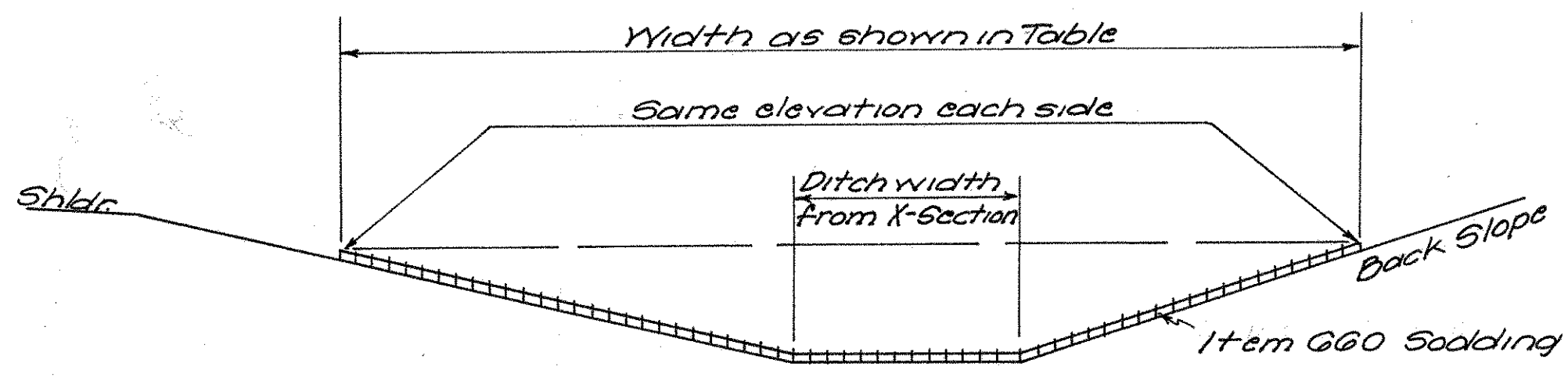
SECTION "C-C"
MODIFIED INLETS AT
STA. 140+00 & 145+00

NOTE: Remove existing M.H. down to where it reaches its full diameter.

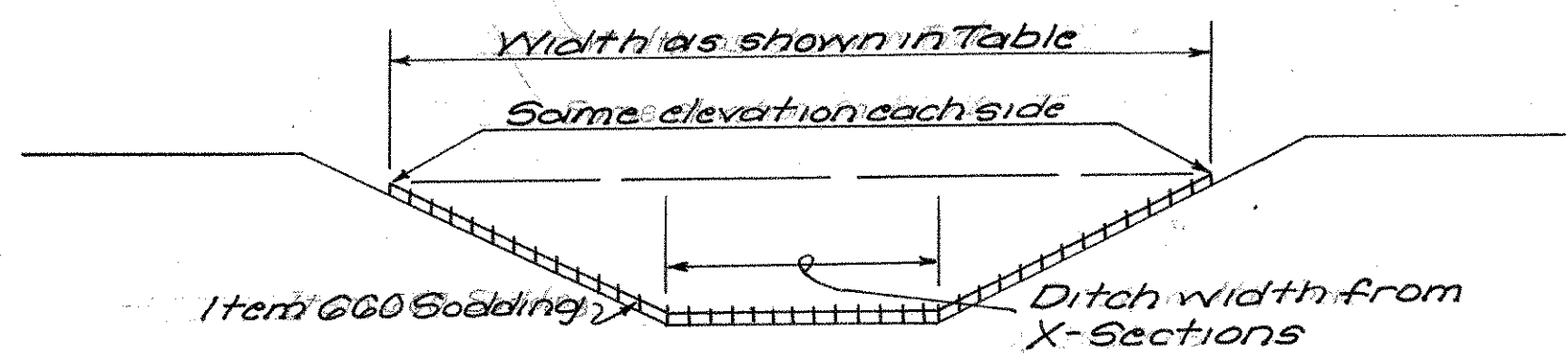


MODIFIED 2-A-6, 2-A-8, 2-A-10 & 2-A-12 INLETS

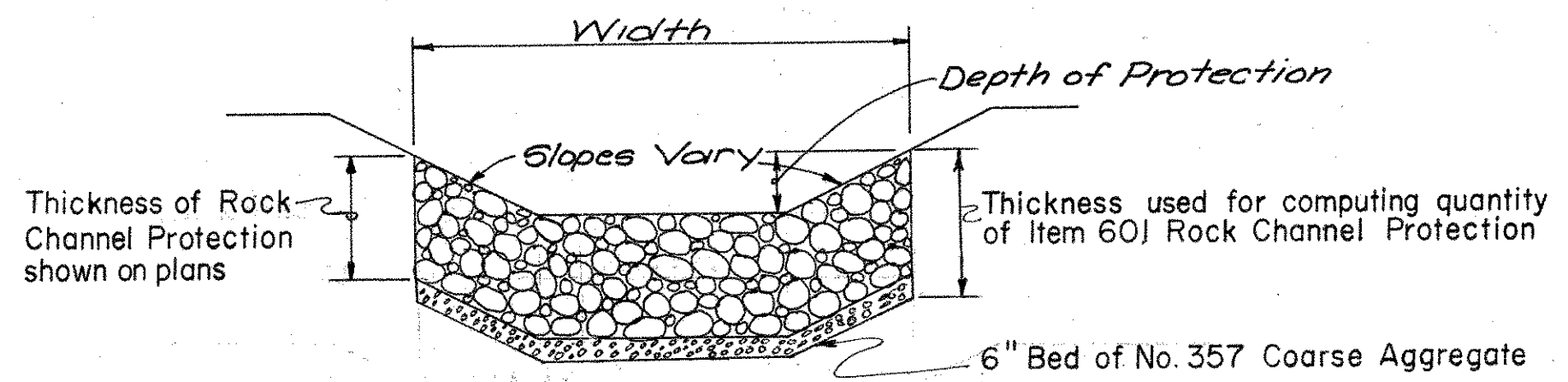
MONTGOMERY COUNTY - MD-75-17.66
 Drawn by T.R. Chkd by WAF 11/68



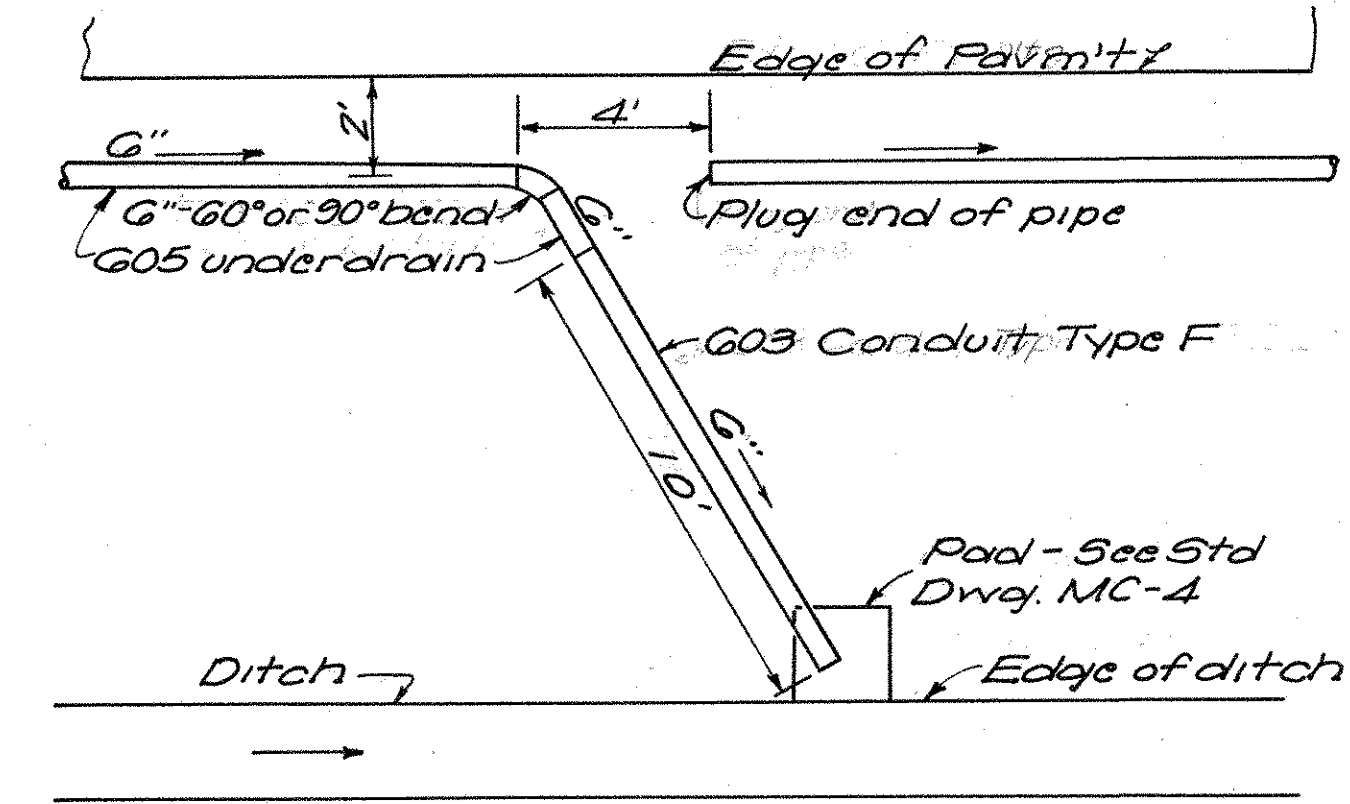
STD. SHOULDER DITCH



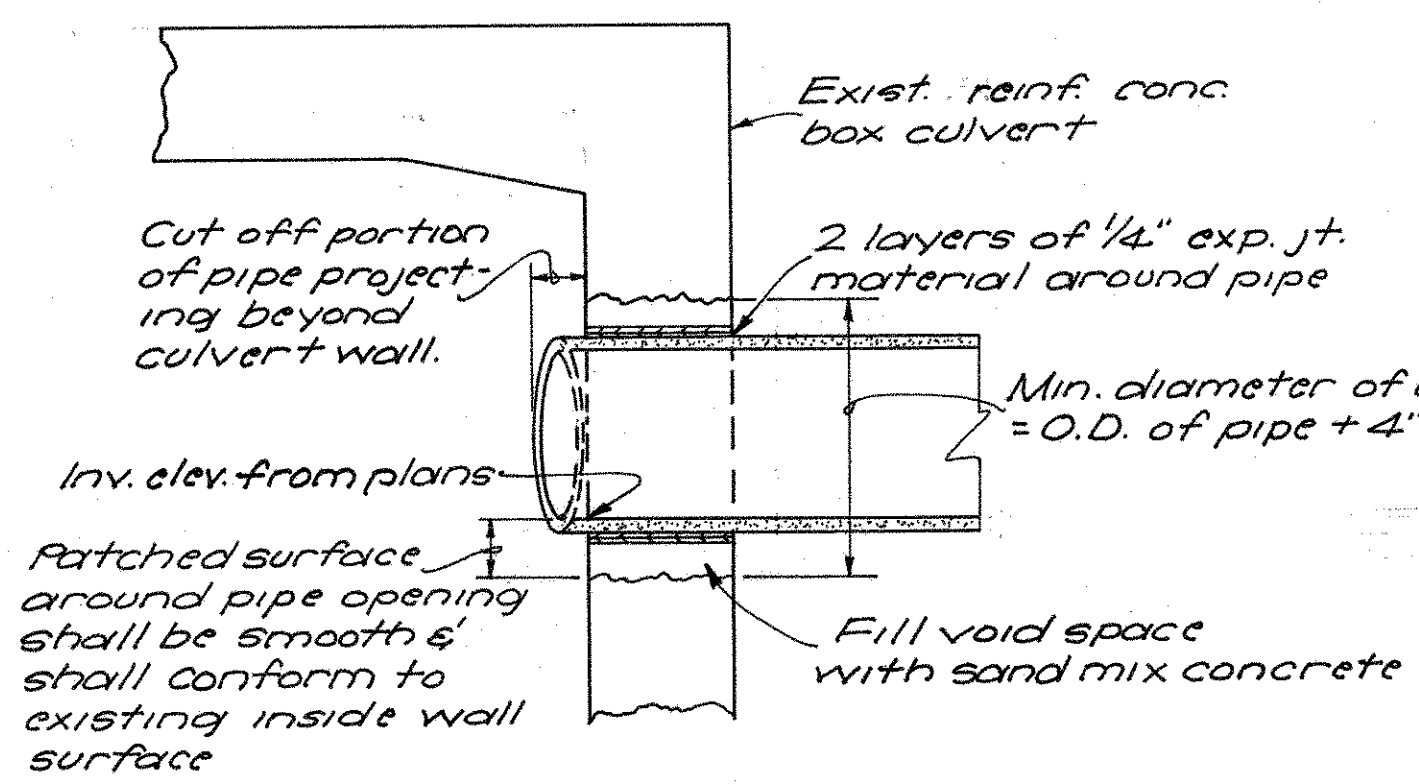
SPECIAL DITCH



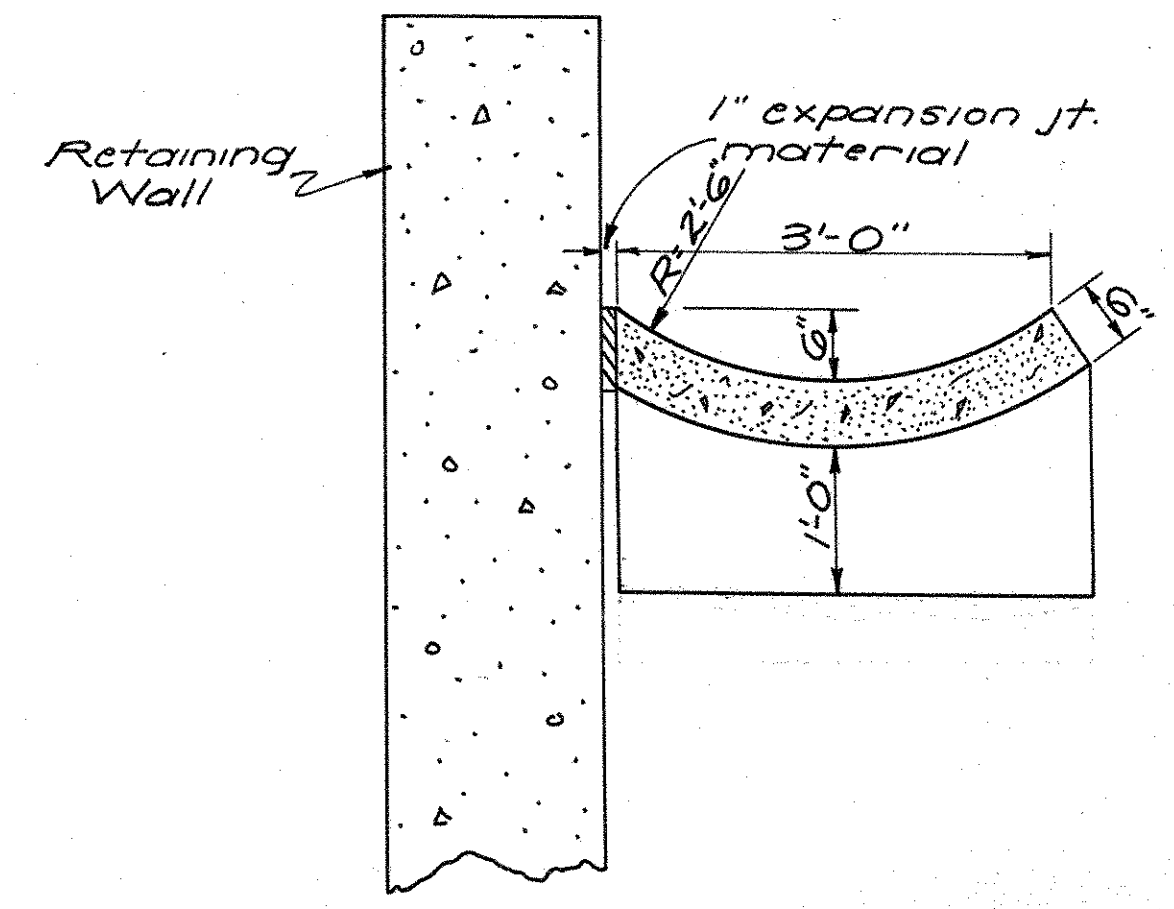
DETAIL OF ROCK CHANNEL PROTECTION



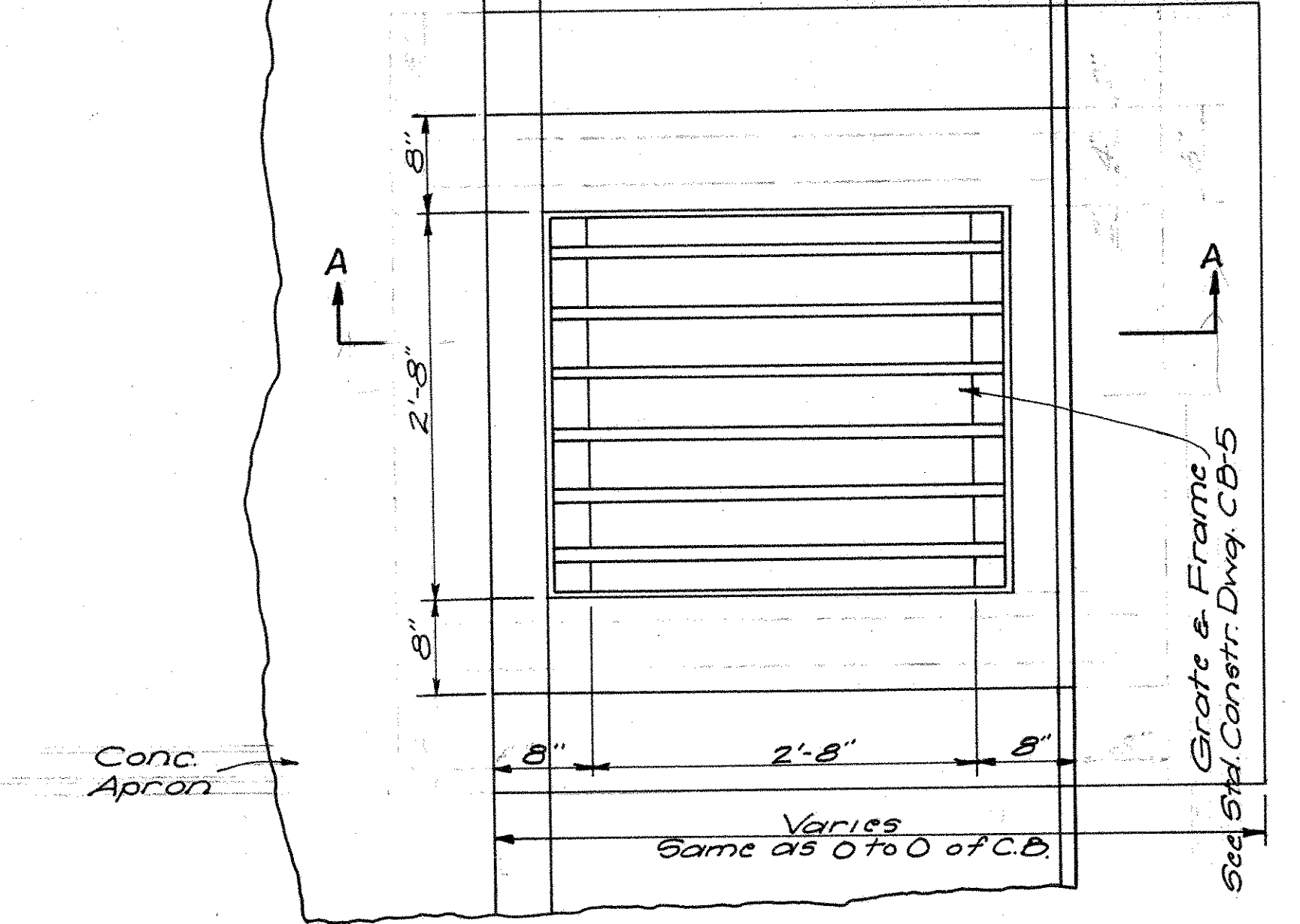
UNDERDRAIN OUTLET TO SIDE DITCH



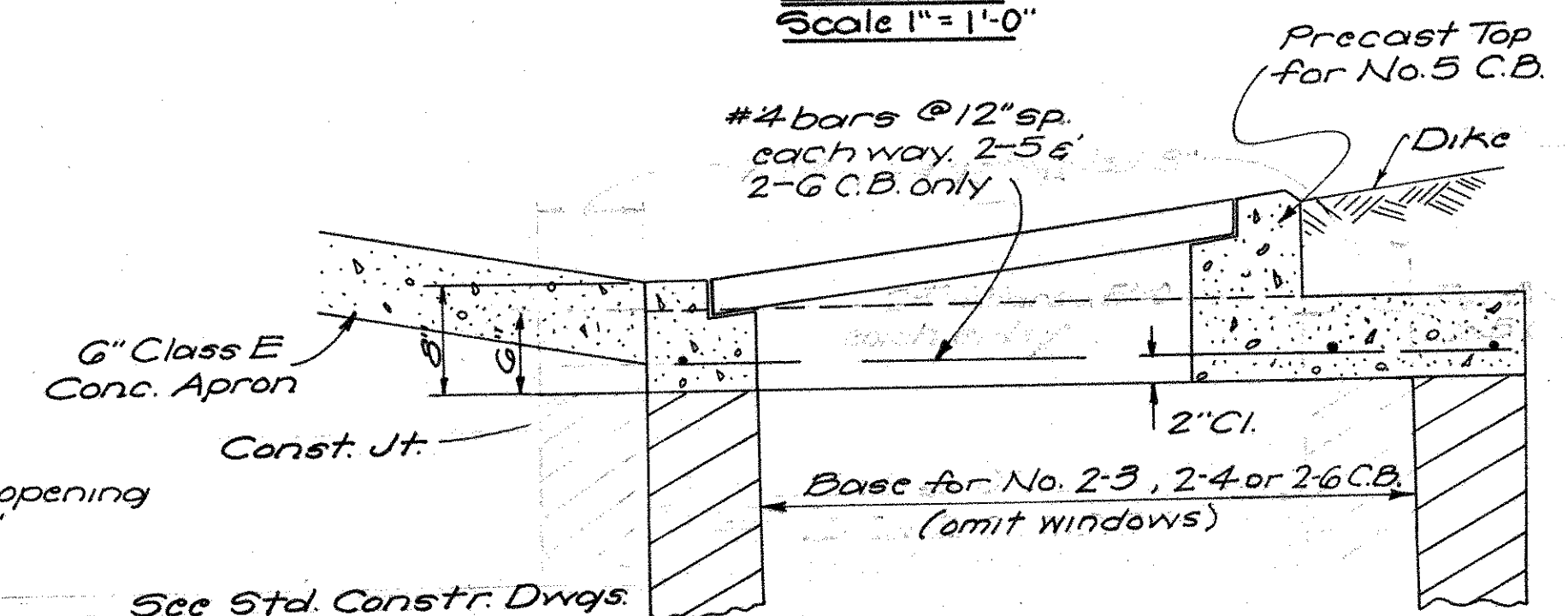
DETAIL PIPE CONNECTION TO EXISTING BOX CULVERT
 No Scale



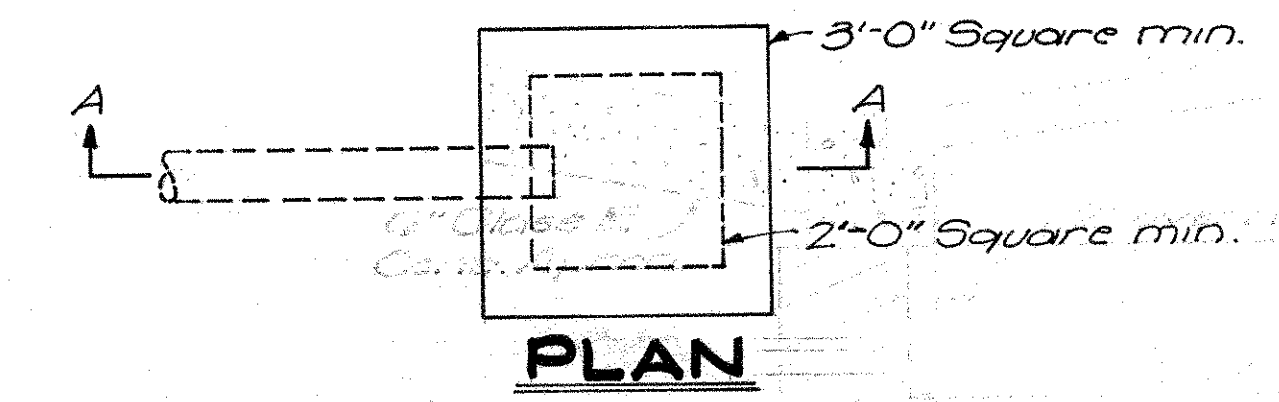
MODIFIED TYPE 2 PAVED GUTTER - PLAN - Y
 Scale 3/4" = 1'-0"



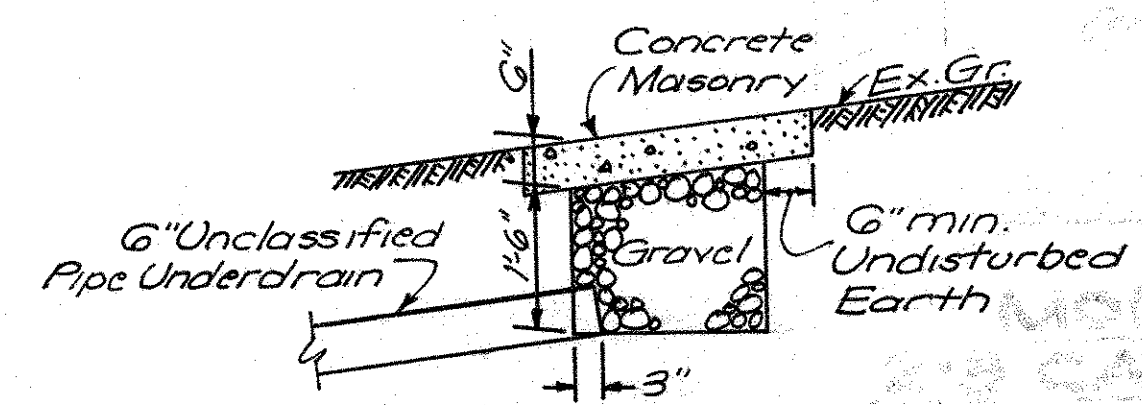
PLAN
 Scale 1" = 1'-0"



SECTION A-A MODIFIED NO. 2-3, 2-4, 2-6 CATCH BASIN WITH NO. 5 C.B. TOP

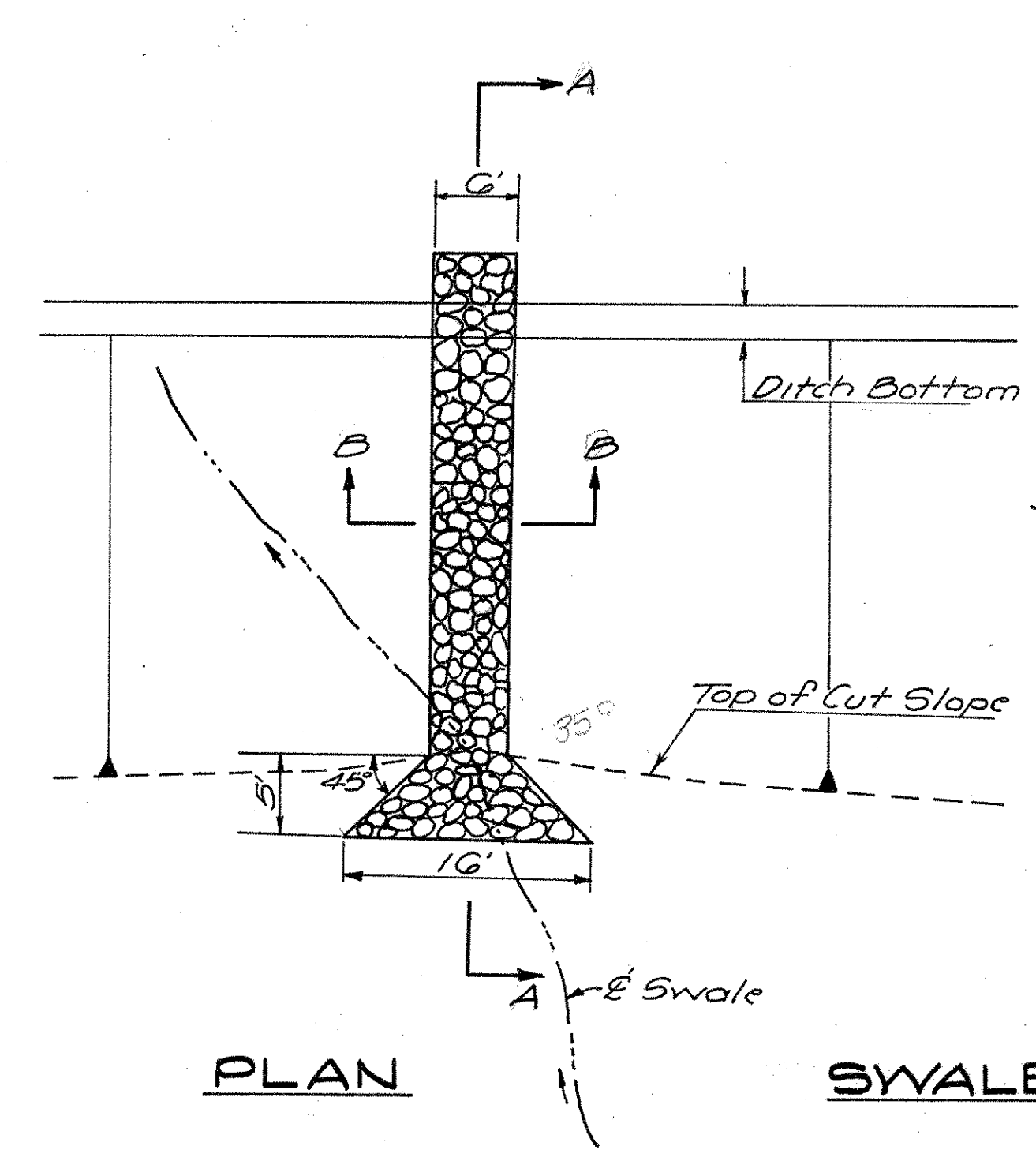


PLAN

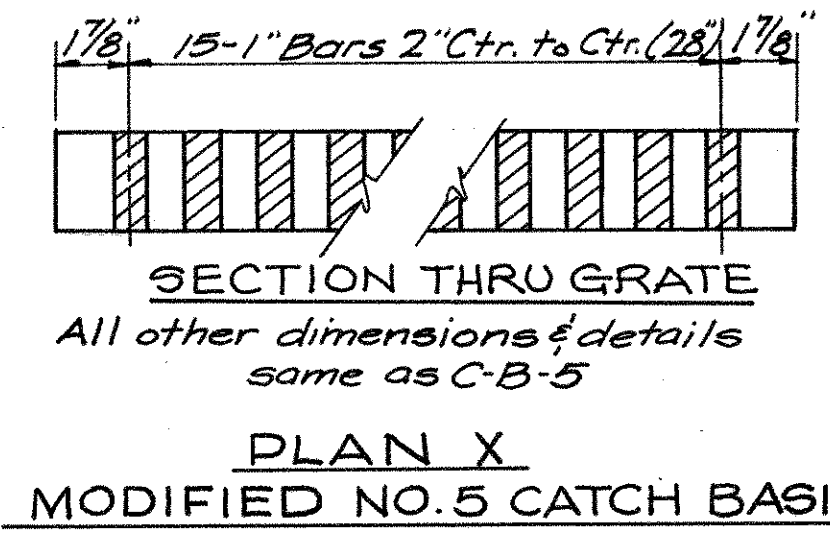
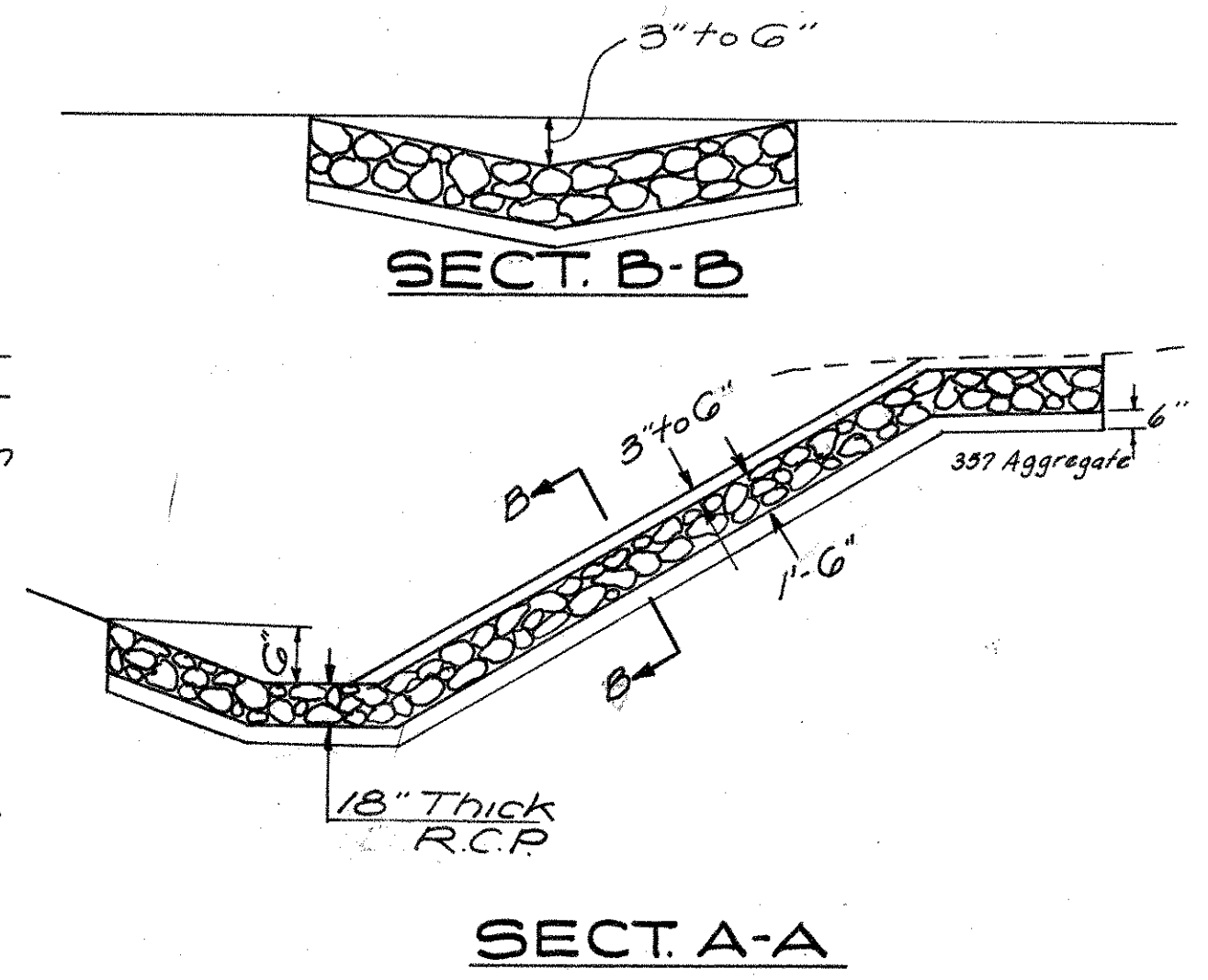


SECTION A-A SPRING DRAIN DETAILS
 Scale 1/2" = 1'-0"

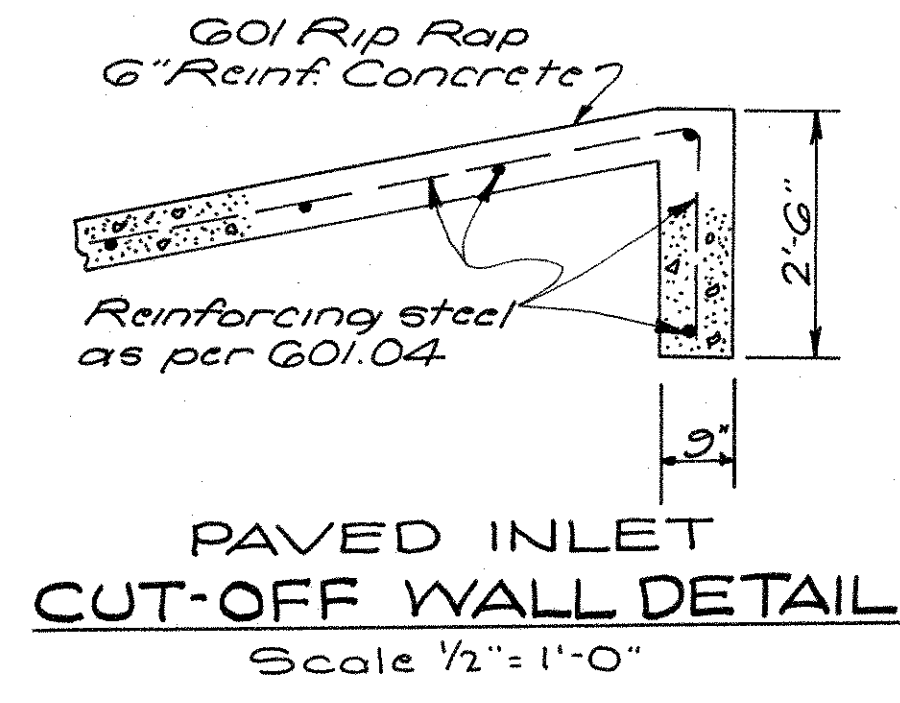
Drawn By T.R. Chkd By WHP



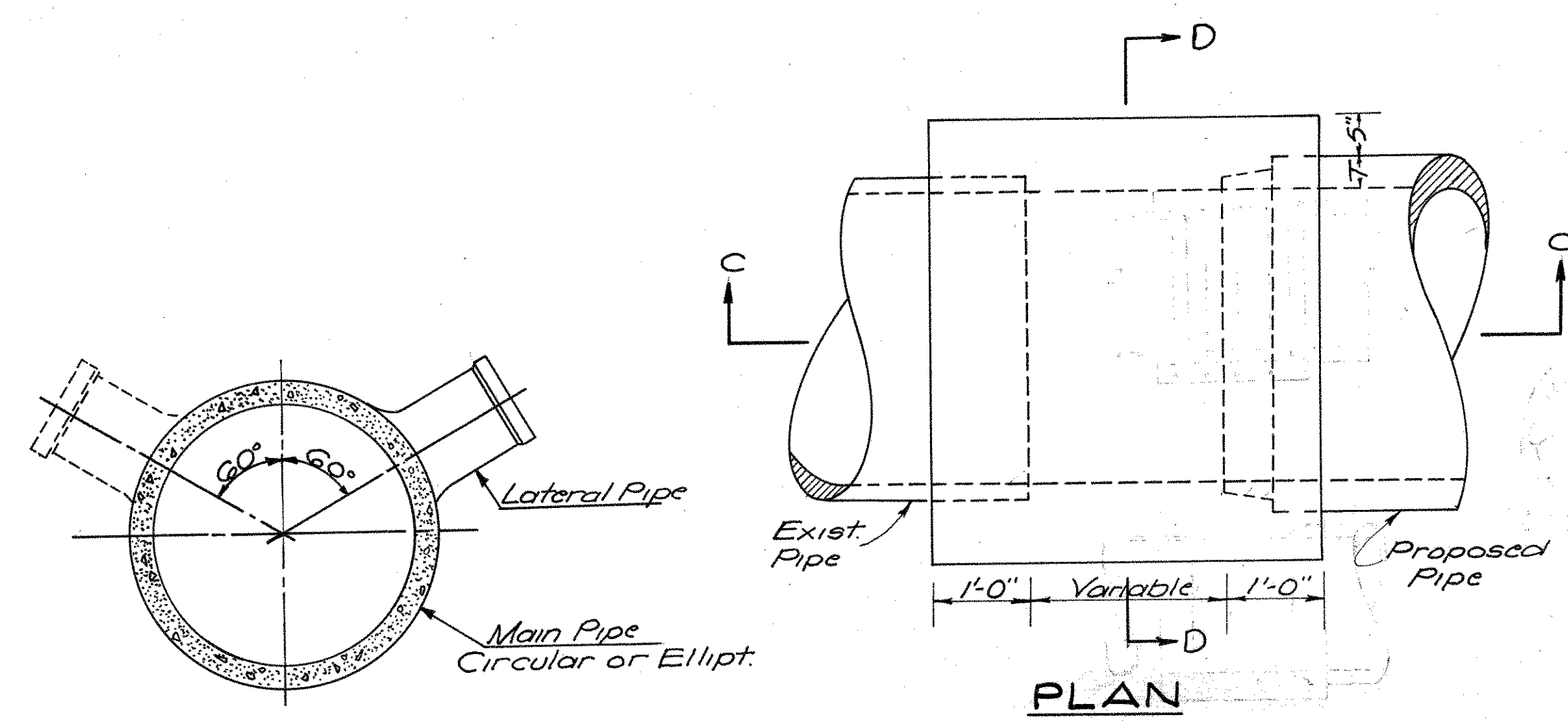
SWALE INTERCEPTOR DETAIL
No Scale



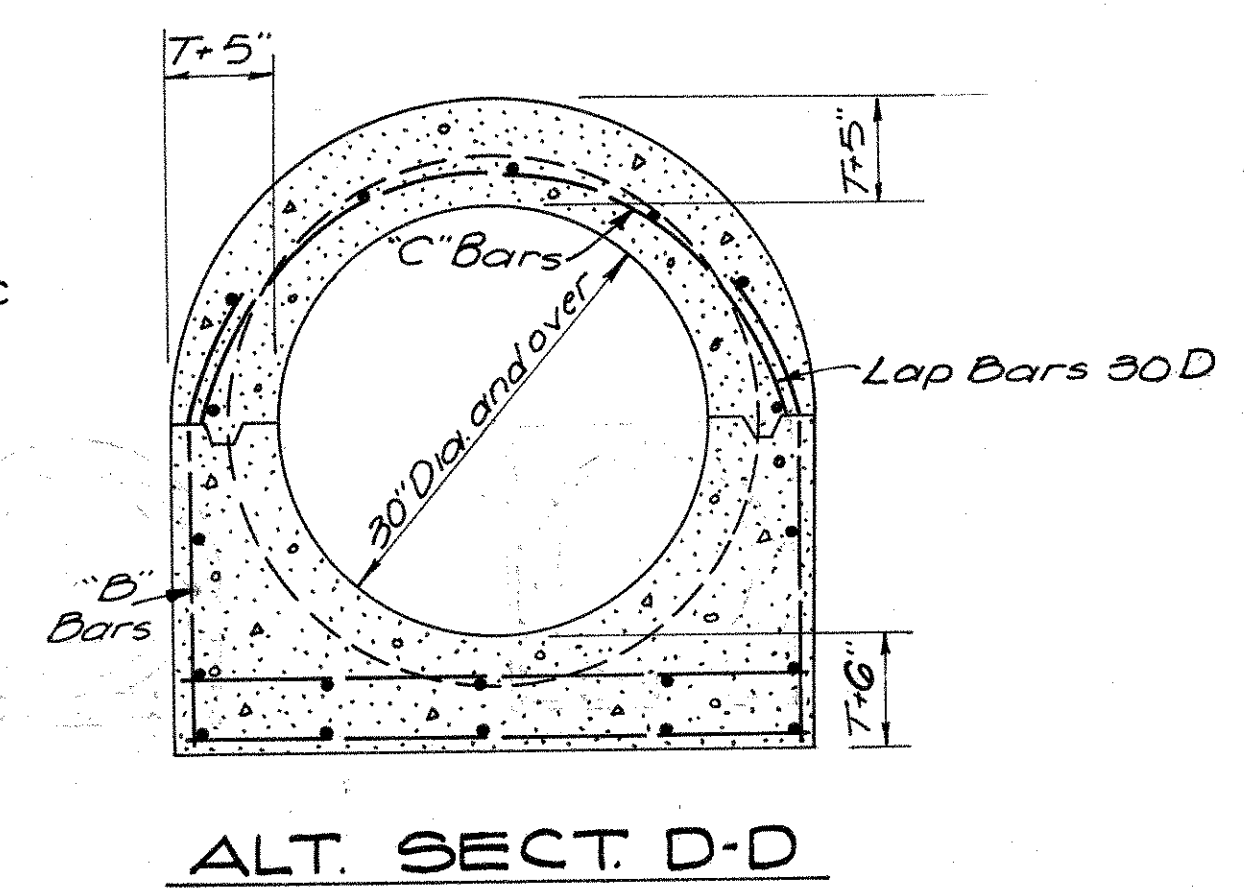
PLAN X
MODIFIED NO. 5 CATCH BASIN



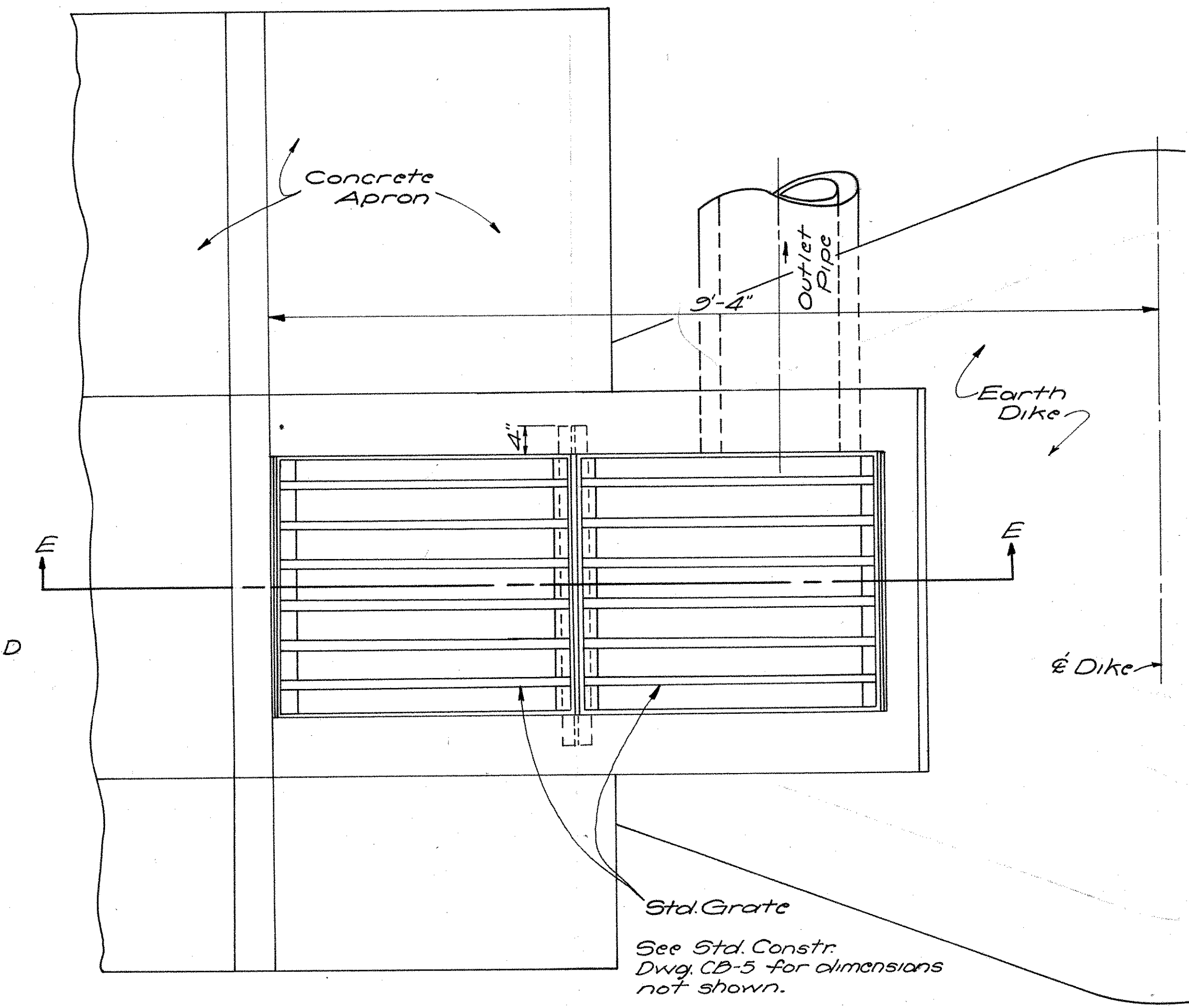
PAVED INLET CUT-OFF WALL DETAIL
Scale 1/2" = 1'-0"



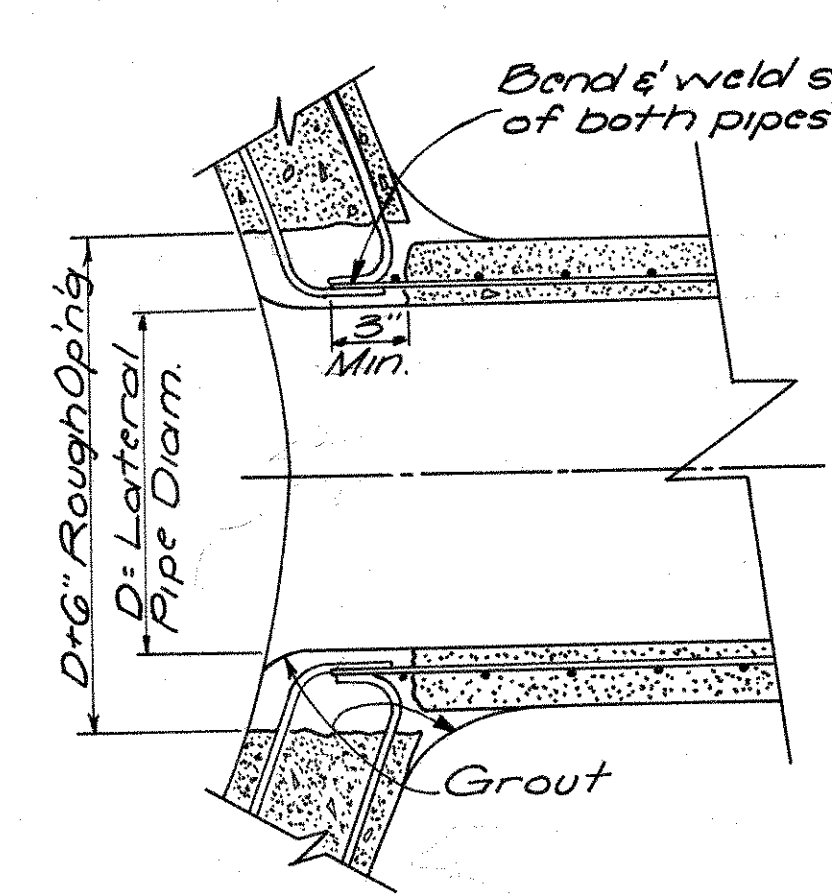
REINFORCED CONCRETE COLLAR
Scale 3/4" = 1'-0"



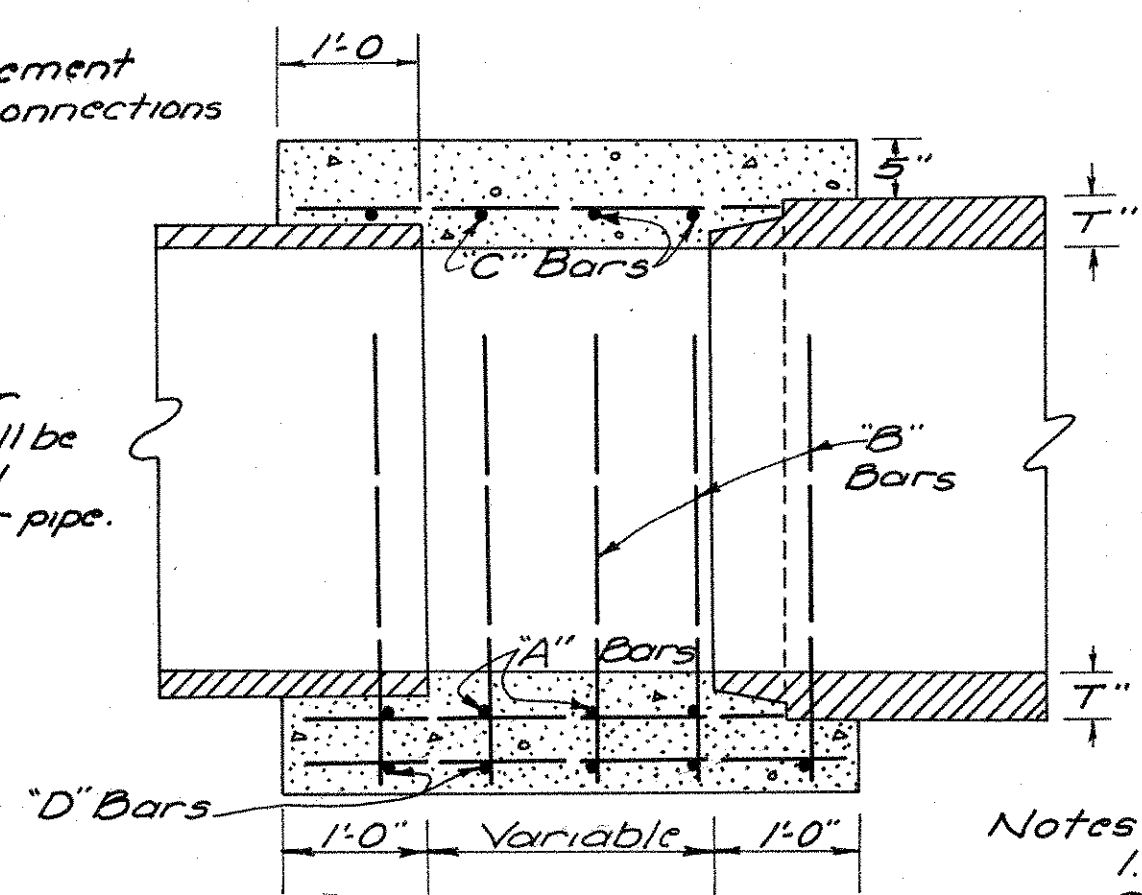
ALT. SECT. D-D



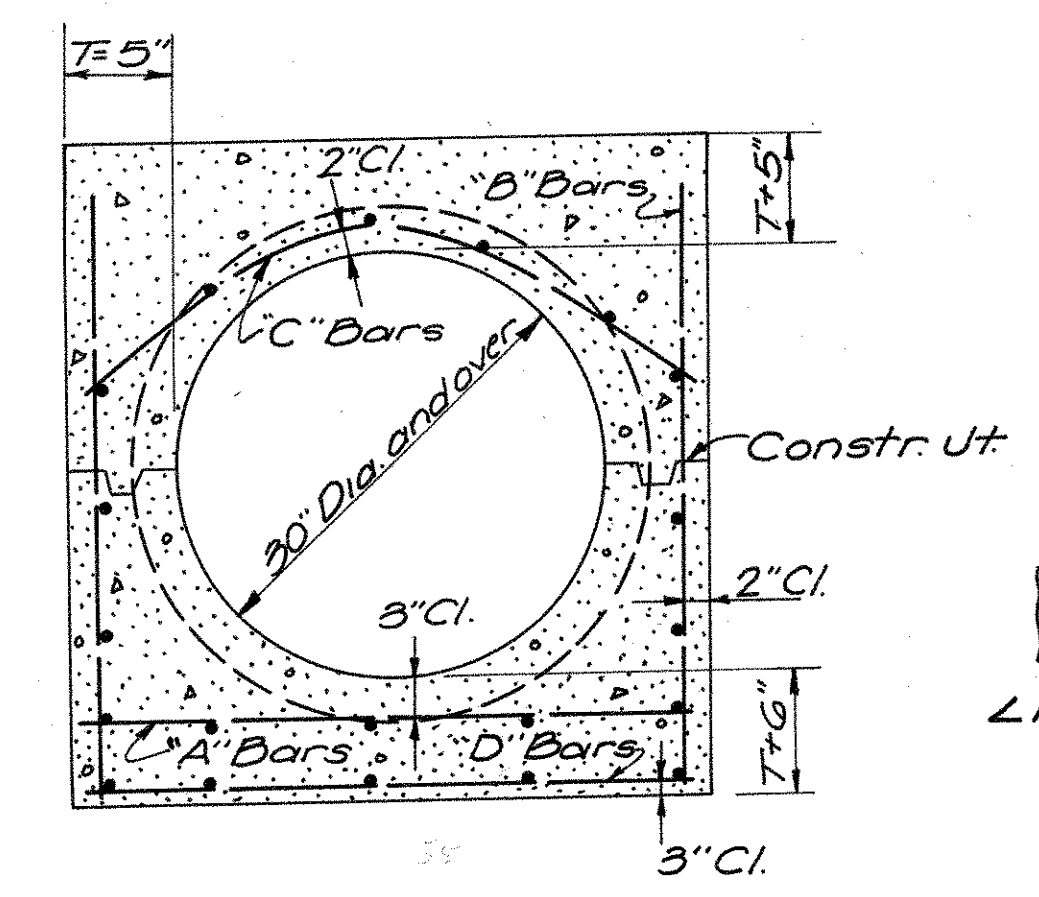
PLAN-Y
MODIFIED NO. 5 CATCH BASIN
WITH DOUBLE GRATE
Scale: 1" = 1'-0"



TYPICAL PREFABRICATED "T" CONNECTION
No Scale

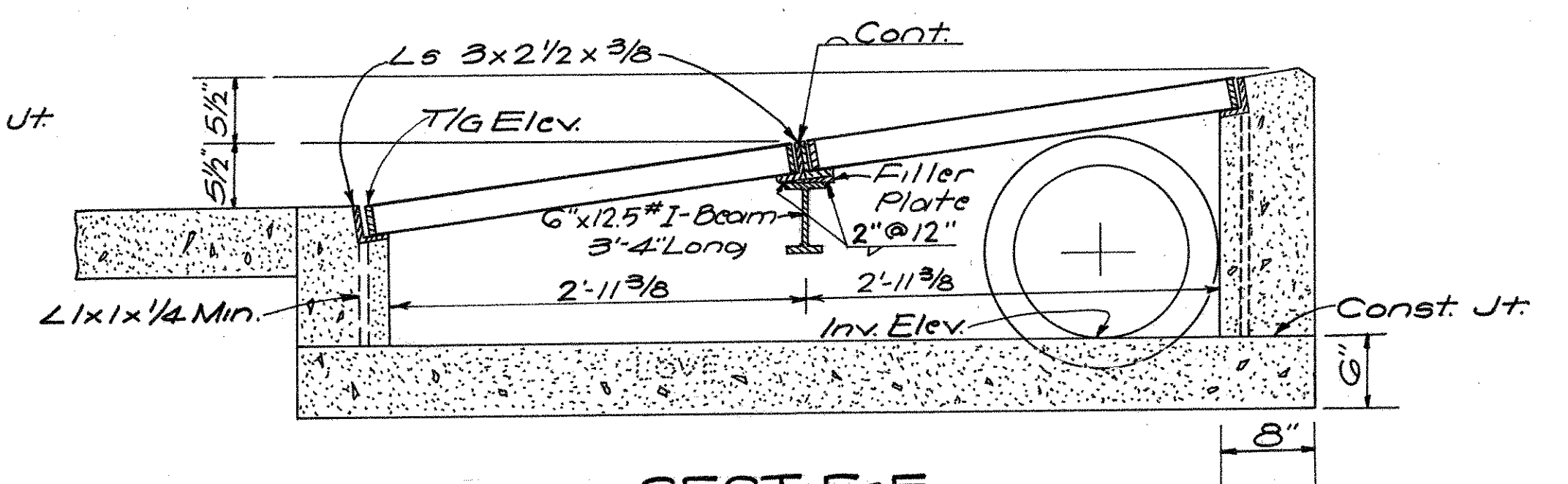


SECT. C-C



SECT. D-D

- Notes:
1. A, B, C, & D bars @ 12" c-c
 2. A, B & C bars #5 for 30" to 60" pipe
#6 for 66" to 78" pipe
#7 for 84" to 108" pipe
 3. D bars #5 for all pipe sizes
 4. Longitudinal bars #5 @ 18" c-c
 4. No less than 3 transverse bars in any gap.



SECT. E-E

CURVE DATA

Dc = 1°30'00"
 RC = 72+61.70
 PI = 77+19.60
 PT = 81+73.14 & Angle
 Point 0°02'13" Lt.

PROPOSED STRUCTURE

TYPE: Continuous reinforced concrete slab with reinforced concrete substructure
 SPANS: 30'-6", 43'-6", 30'-6"
 ROADWAY: 117'-0" face to face of parapets including a 3' median barrier and 2'-1-0" safety curbs.
 LOAD FREQUENCY: CF=2000(5T) (Adequate for AASHTO alternate loading)
 SKEW: 0°-00'-00"
 WEARING SURFACE: 1" Monolithic Concrete
 APPROACH SLAB: 20' long (AS-1-67)
 ALIGNMENT: Tangent

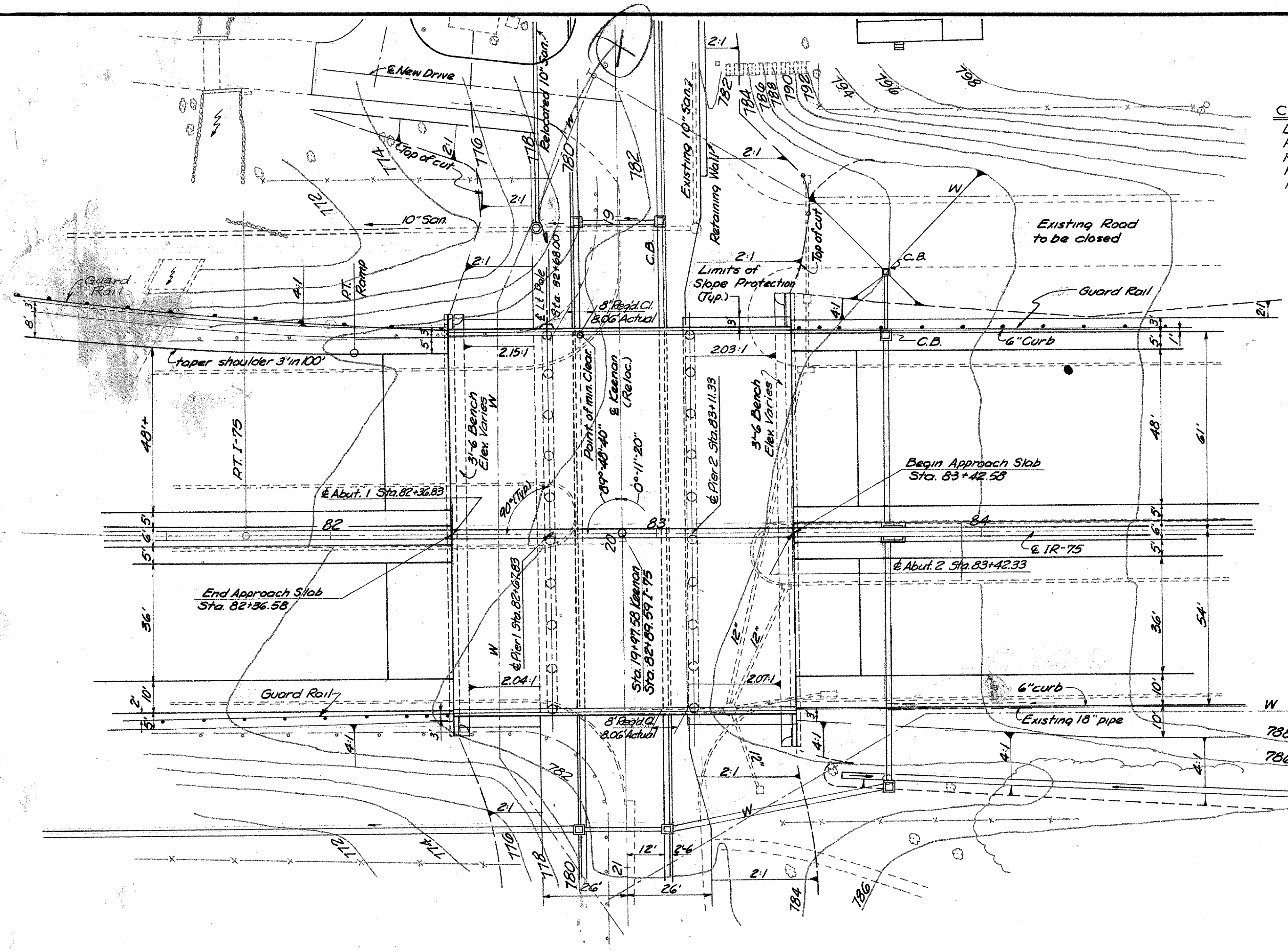
NOTE: 4 lanes of traffic to be maintained during construction of bridge. Keenan Ave. traffic to be detoured during construction of Keenan Ave.

AVERAGE DAILY TRAFFIC

A.D.T. (1975) 40500*
 Type "B" (Estimated) 2015

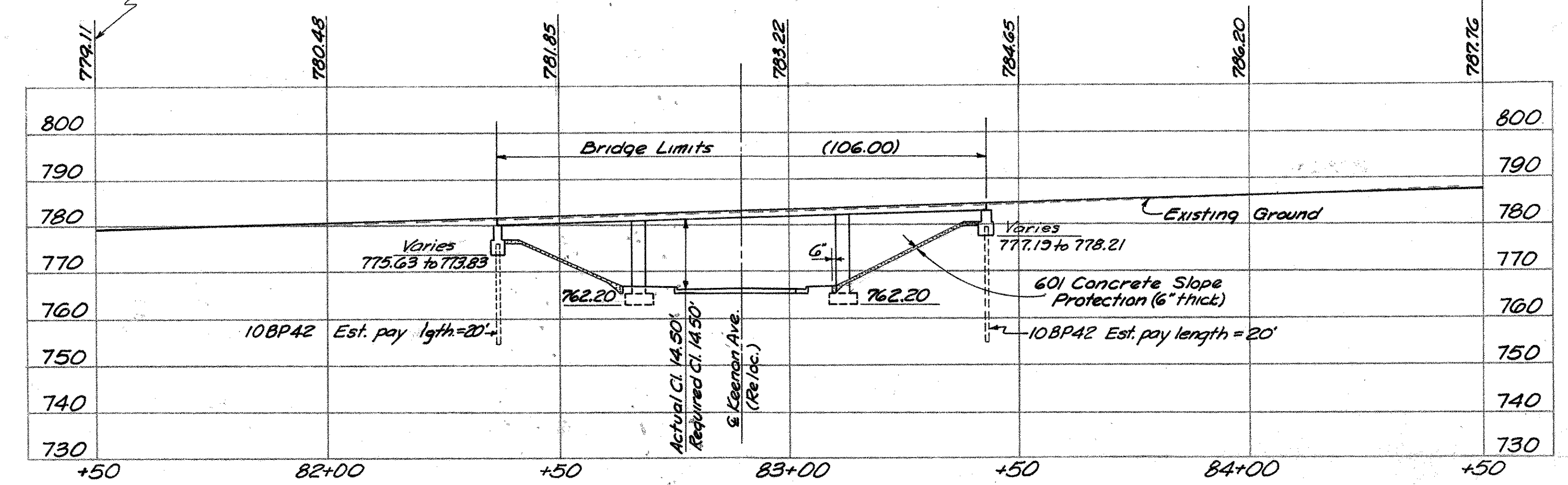
* From an Engineering Report prepared by Division 8, March, 1962. Traffic count made by Bureau of Planning and Survey

B.M. 2" x 2" Cut in E. side of ditch inlet structure. 79' Rt. of Sta. 83+32

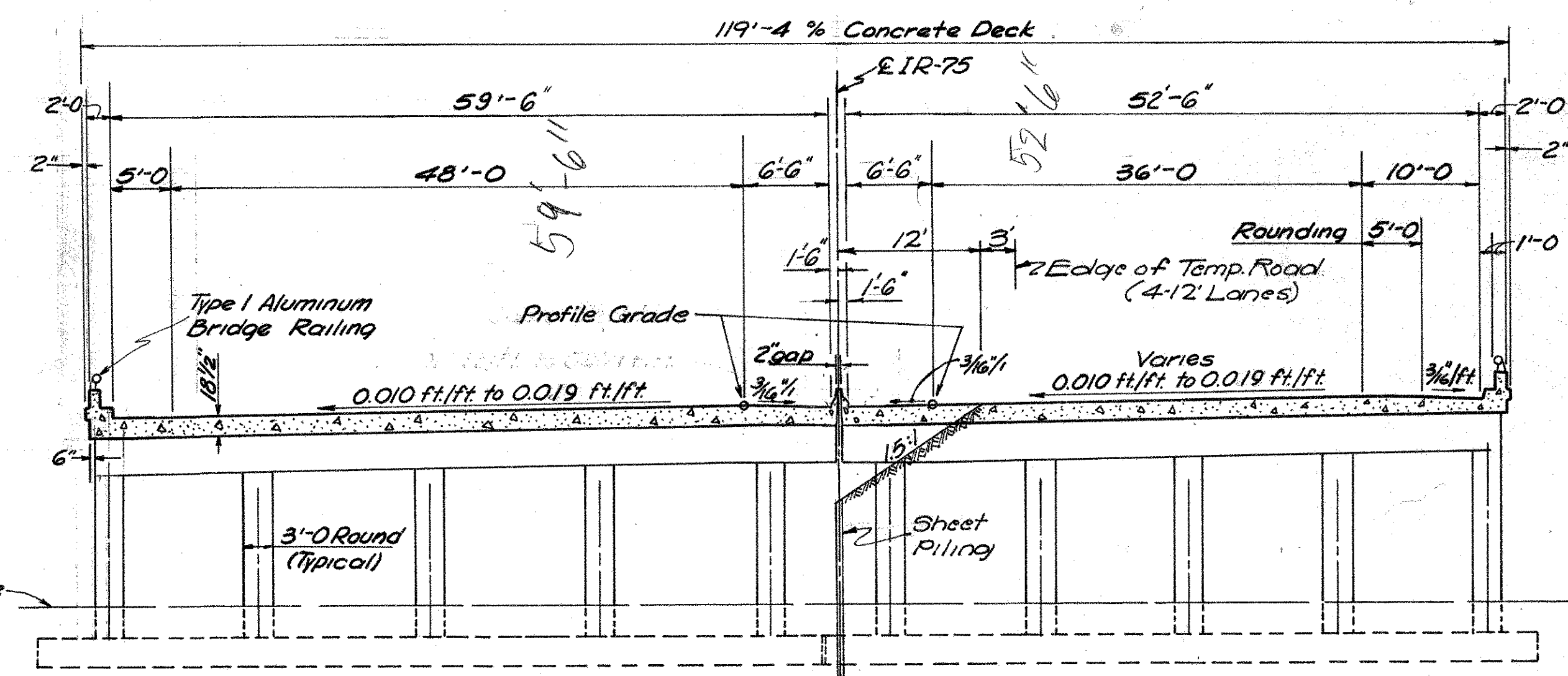


PLAN

NOTE: Profile grades shown are 8' Rt. & Lt. of I-75.



PROFILE



TYPICAL SECTION THRU BRIDGE

Note: West half of bridge to be built first. Four lanes of traffic to be carried on east half of I-75 during this period. Use sheet piling as indicated.

NOTE: All piers and abutments are parallel and 90° to proposed I-75.

Grades worked out graphically see sheet 10

SHAW, LENZ & ASSOCIATES ENGINEERS CINCINNATI OHIO

SITE PLAN

BRIDGE NO. MOT-75-1783
 IR-75 OVER KEENAN AVENUE
 STA. 82+36.58 to STA. 83+42.58

PRESENT SURVEYOR	TOPO DRAWN	PROPOSED DESIGNED	WORK DRAWN	CHECKED	REVISED
B. H. S.	W. B. S.	W. B. S.	T. J. P.	R. L. G.	6-27-66

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	LEFT BRIDGE				RIGHT BRIDGE			
				ABUTMENTS	PIERS	SUPER-STRUCTURE	GENERAL	ABUTMENTS	PIERS	SUPER-STRUCTURE	GENERAL
503	587	Cu. Yds.	Excavation for Structures, unclassified	144	154			132	157		
505	Lump	Lump Sum	First Test Pile	Lump							
507	800	Lin. Ft.	Steel Piles (10 BP42)	420				380			
509	229,666	Lbs.	Reinforcing Steel	8,388	24,124	89,227		7,519	20,631	79,777	
511	758	Cu. Yds.	Class "C" Concrete - Superstructure			401				357	
511	125	Cu. Yds.	Class "C" Concrete - Piers above Footings		63				62		
511	130	Cu. Yds.	Class "C" Concrete - Abutments	68				62			
517	212	Lin. Ft.	Bridge Railing, Type 1			106				106	
518	68	Cu. Yds.	Porous Backfill	35				33			
518	24	Each	Scuppers, 6" cast iron pipe as per plan			12				12	
512	6	Lin. Ft.	Premolded Sealing Strip	3				3			
516	38	Sq. Ft.	1" Preformed Expansion Joint Filler	13		6		13		6	
601	875	Sq. Yds.	6" thick Concrete Slope Protection				462				413
808	808	Unit of Units	Water-reducing, set-retarding admixture		26	401			24	357	
511	115	Cu. Yds.	Class "C" Concrete - Pier Footings		61				54		
503	Lump	Lump Sum	Cofferdams, cribs and sheeting				Lump				
625			See lighting plan.								

GENERAL NOTES

REFERENCE shall be made to the following
Standard Drawings: DR-1-65, sheet 1, revised 11-24-65
AS-1-67 Dated 6-18-69

Supplemental Specifications: 808 dated 1-1-69
836 dated 6-17-69

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57 together with current revisions thereof.

DESIGN LOADING: CF = 2000 (57)

CONCRETE: Class "C" basic unit stress = 1333 p.s.i. for superstructure.
Class "C" basic unit stress = 1133 p.s.i. for substructure.

REINFORCING STEEL: ASTM, A615, A616, A617, Deformed Intermediate or Hard Grade. Basic unit stress = 20,000 p.s.i.
Spiral reinforcement shall be plain, A306 or A499

MACHINE FINISH: The concrete bridge slab is to be finished by the use of a finishing machine.

FOUNDATION BEARING PRESSURE: Pier footings are designed for a maximum bearing pressure of 3 tons per square foot.

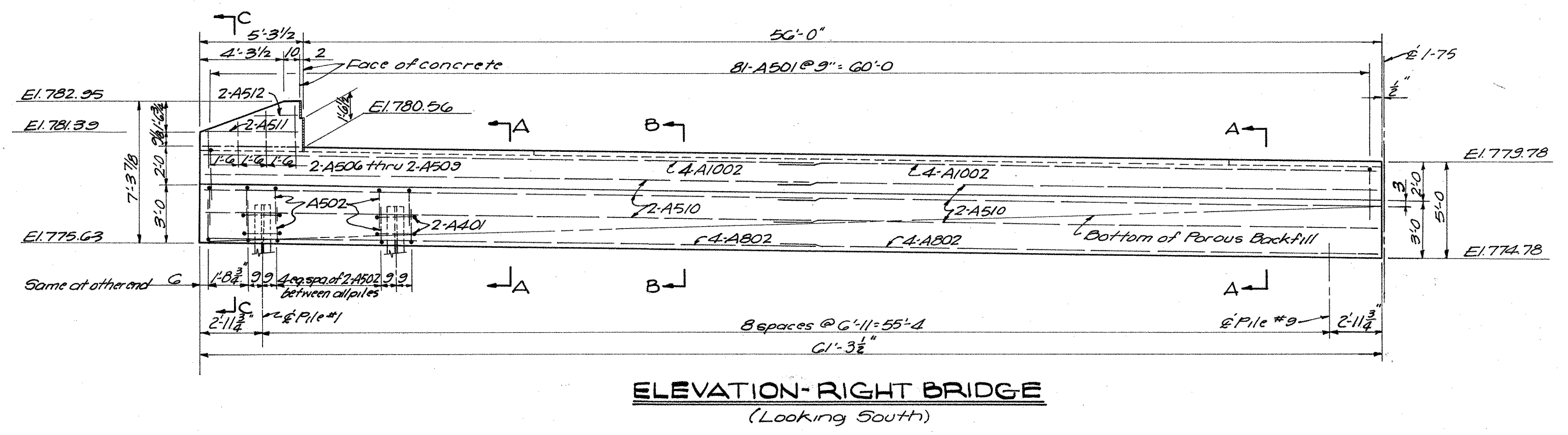
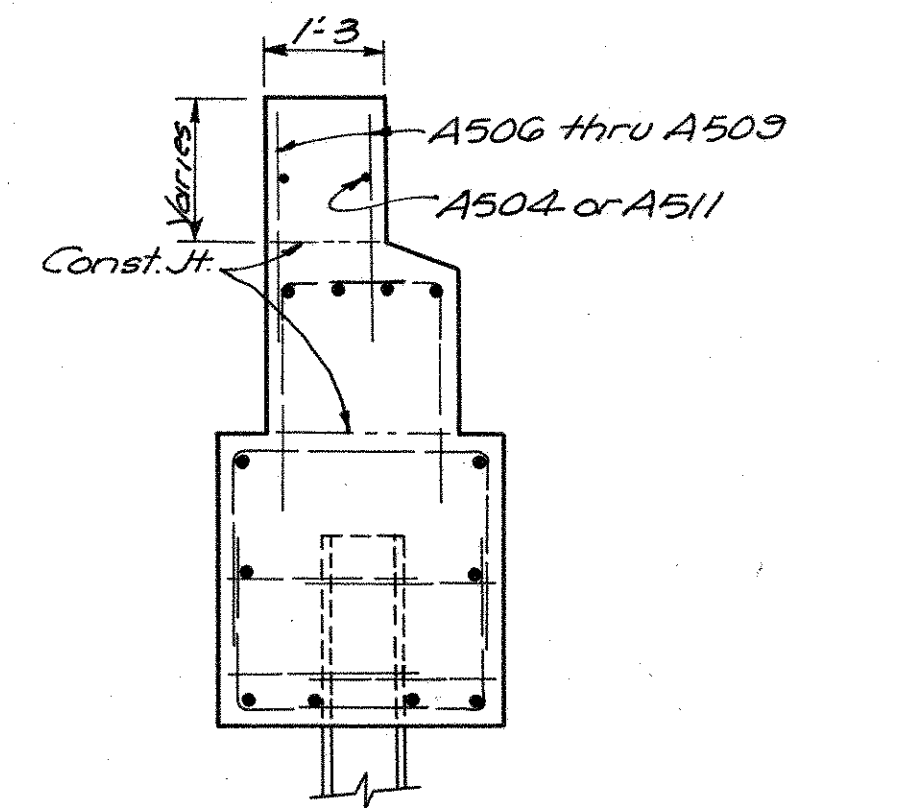
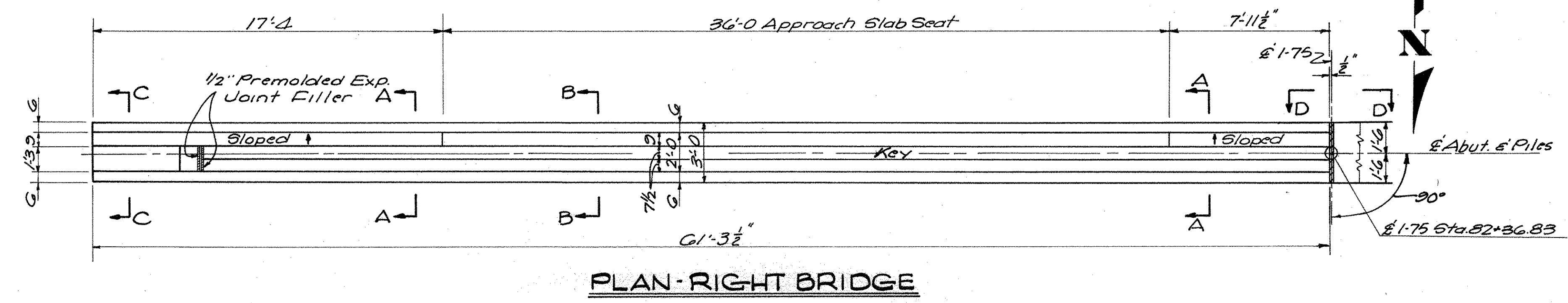
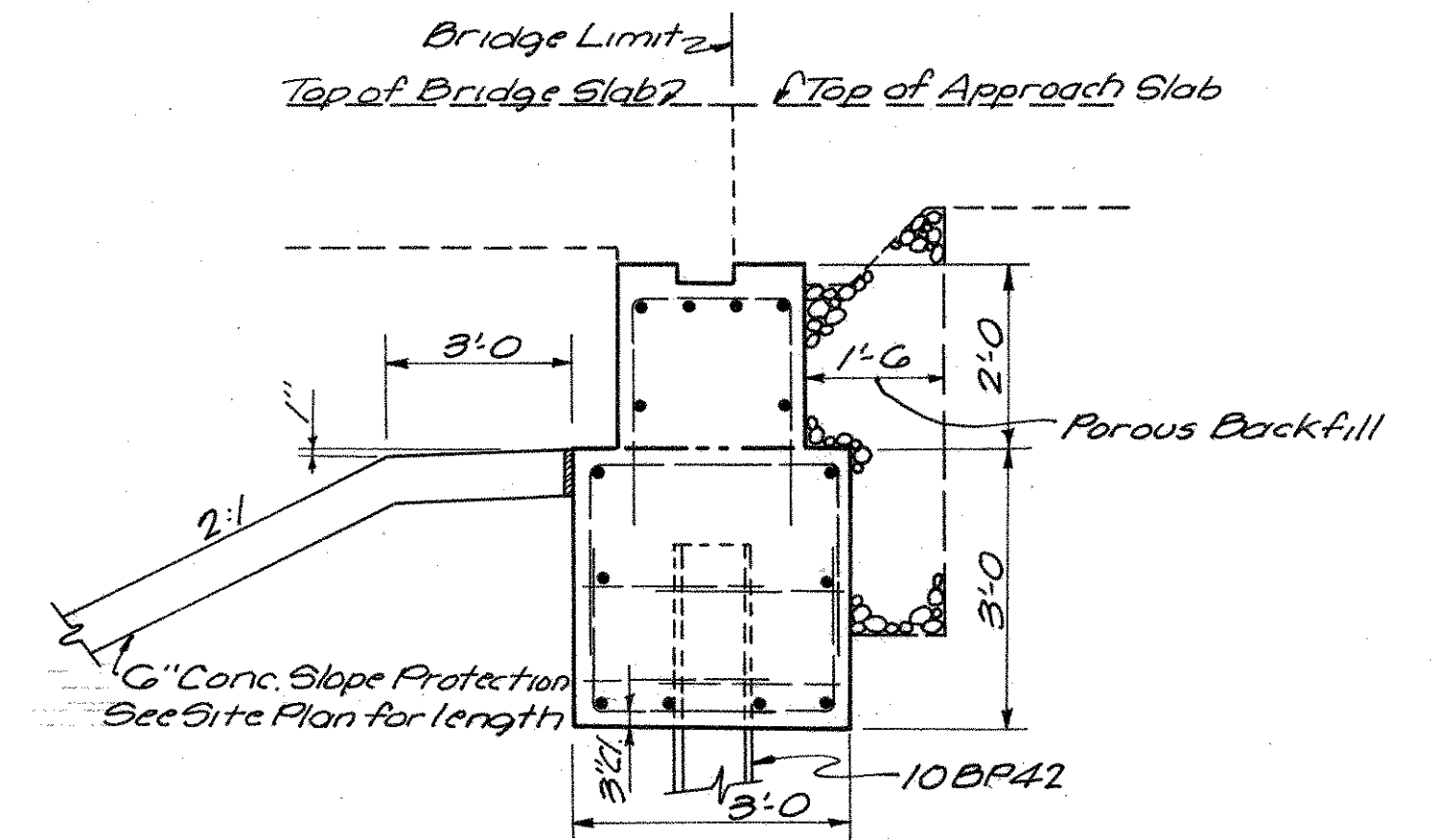
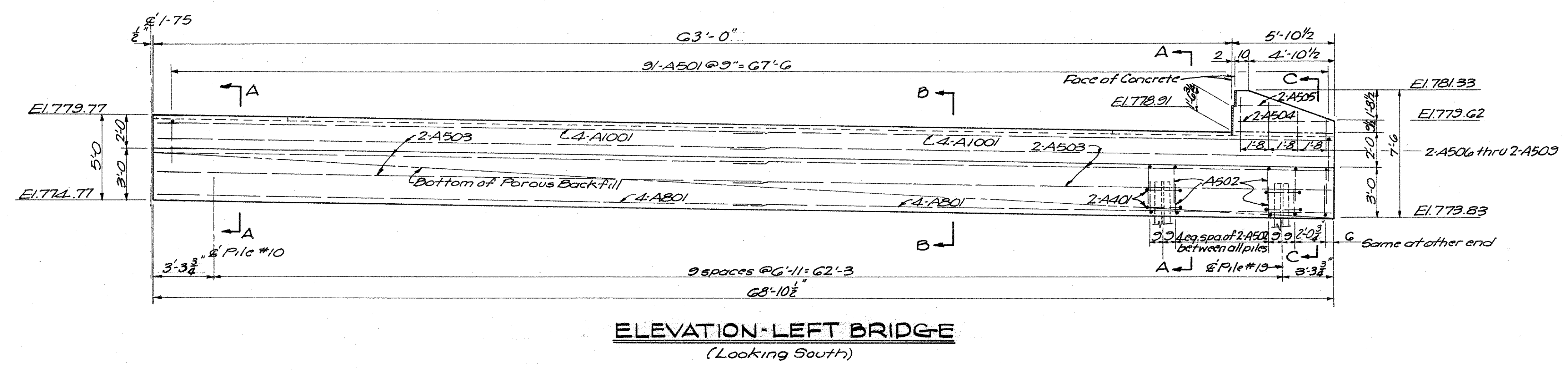
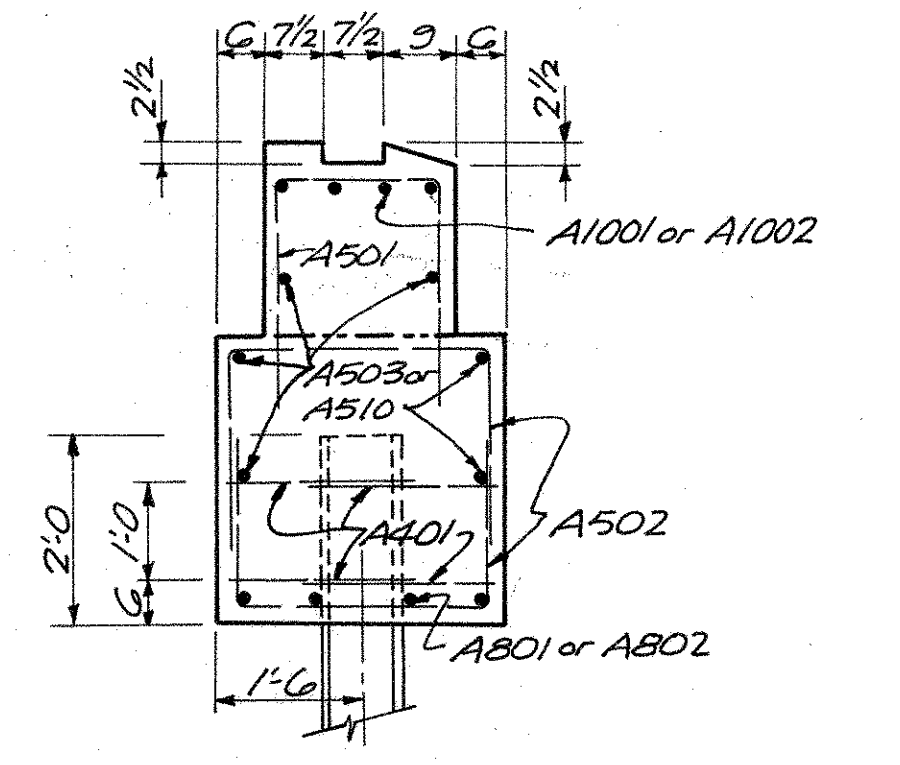
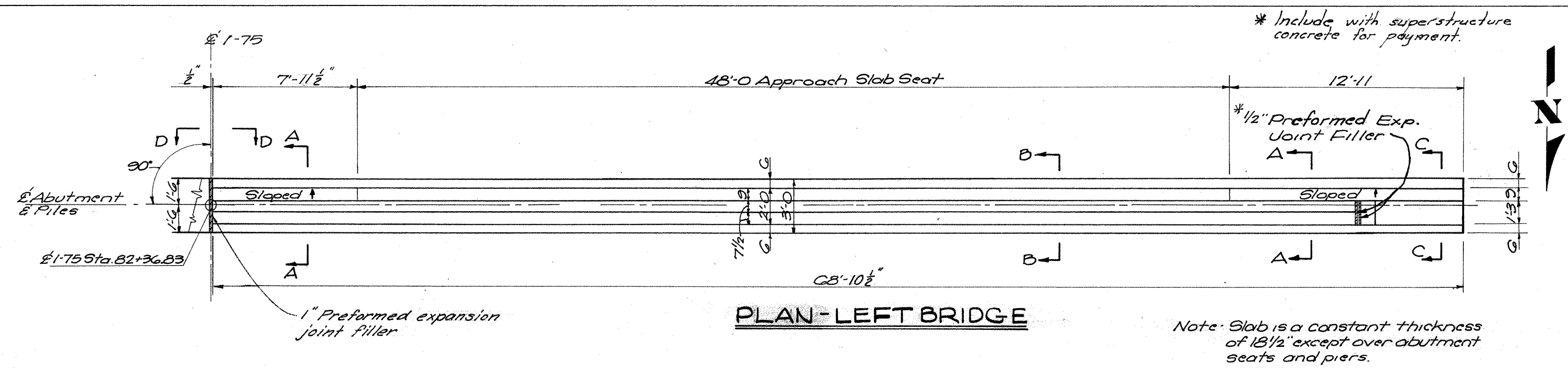
FIRST TEST PILE: Payment will be made for only one first test pile. It may be driven for either the Right or Left bridge.

PILES shall be driven to a minimum bearing capacity of 32 tons per pile for the abutments.

UTILITY LINES: All expenses involved in relocating the affected utility lines shall be borne by the owners. The Contractor and the Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

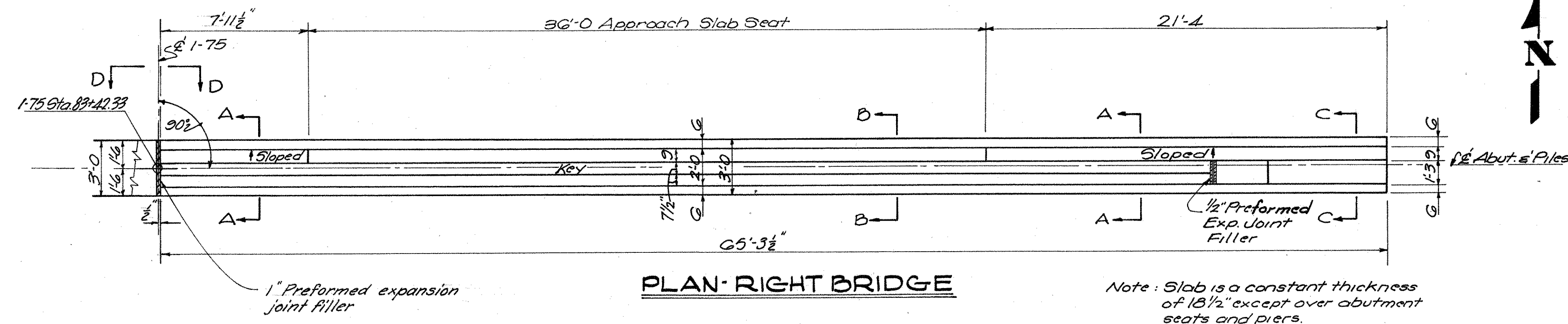
MAINTENANCE OF TRAFFIC: Four lanes of traffic are to be maintained on U.S. 25 during construction of the bridges. Keenan Avenue traffic shall be detoured during construction of Keenan Avenue.

SHAW, LENZ & ASSOCIATES ENGINEERS		2/11
CINCINNATI		OHIO
ESTIMATED QUANTITIES & GENERAL NOTES		
BRIDGE NO. M0T-75-1783 1R-75 OVER KEENAN AVENUE		
MONTGOMERY COUNTY STA. 82+36.58 to STA. 83+42.58		
DESIGNED	DRAWN	TRACED
W.B.S.	R.R.L.	W.T.R.
		R.J.L. 6-28-66
CHECKED	REVIEWED	REVISED



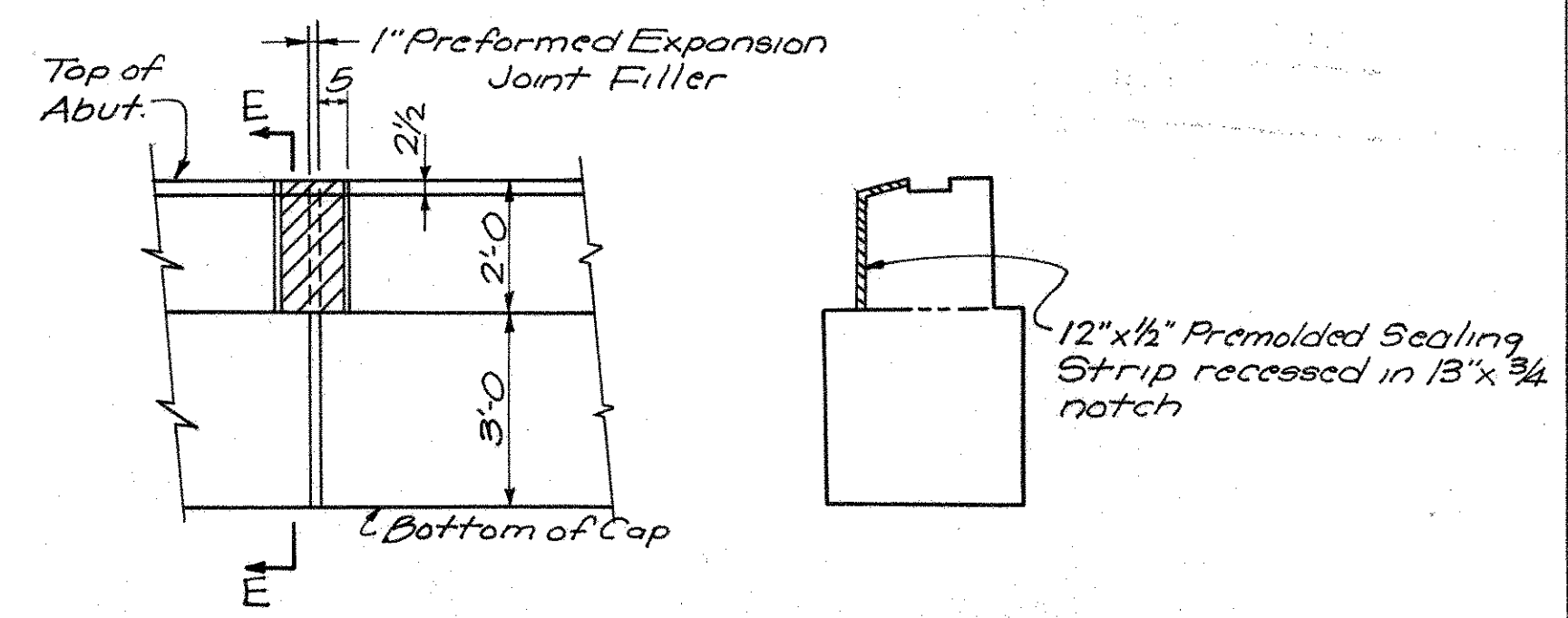
NOTE:
For Section D-D and additional Notes,
see Sheet 4.

SHAW, LENZ & ASSOCIATES ENGINEERS		3/11
CINCINNATI		OHIO
ABUTMENTS NO. 1		
BRIDGE NO. MOT-75-1783		
IR-75 OVER KEENAN AVENUE		
MONTGOMERY COUNTY STA 87+36.58 to STA 83+42.58		
DESIGNED	DRAWN	TRACED
W.B.S.	R.R.L.	W.T.R.
CHECKED	REVIEWED	REVISED
R.J.L.	G.28-66	

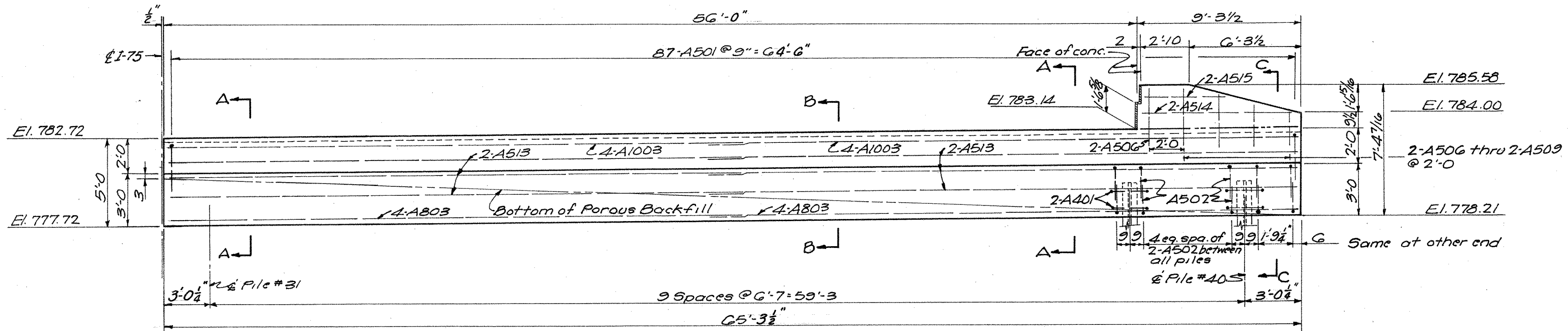


PLAN - RIGHT BRIDGE

Note: Slab is a constant thickness of 18 1/2" except over abutment seats and piers.



SECTION D-D **SECTION E-E**



ELEVATION - RIGHT BRIDGE

TOP OF PILE ELEV.

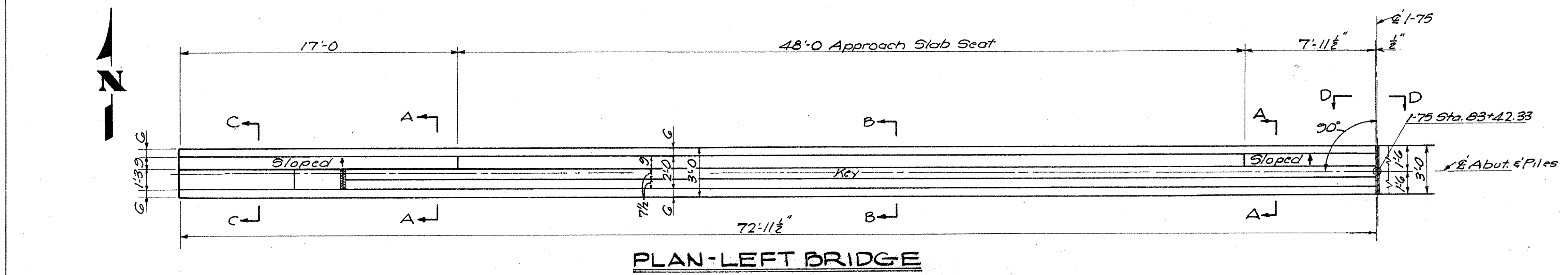
ABUTMENT NO. 1		ABUTMENT NO. 2	
Pile No.	Top of Pile Elev.	Pile No.	Top of Pile Elev.
1	777.58	20	779.21
2	777.49	21	779.26
3	777.39	22	779.31
4	777.30	23	779.36
5	777.20	24	779.41
6	777.11	25	779.46
7	777.01	26	779.51
8	776.91	27	779.55
9	776.82	28	779.60
10	776.73	29	779.65
11	776.64	30	779.70
12	776.54	31	779.75
13	776.44	32	779.80
14	776.35	33	779.85
15	776.25	34	779.89
16	776.16	35	779.94
17	776.06	36	779.99
18	775.97	37	780.04
19	775.87	38	780.09
		39	780.14
		40	780.19

- NOTES:**
- For Sections A-A, B-B, & C-C, see Sheet 3
 - Porous backfill shall extend upward to the approach slab and to the surface of the earth shoulders and outward to the surface of the embankment slopes. Excavation therefore, in excess of that required for construction of the footing, shall be considered as paid for in the bid price per cu. yd. paid for porous backfill.
 - Excavation quantity includes the removal of material bounded on the bottom by the bottom plane of the footing, on the top by the surface of the roadway cut and the removal of material above the level of the earth bench.
 - Falsework for bridge slab may be supported on the footing.

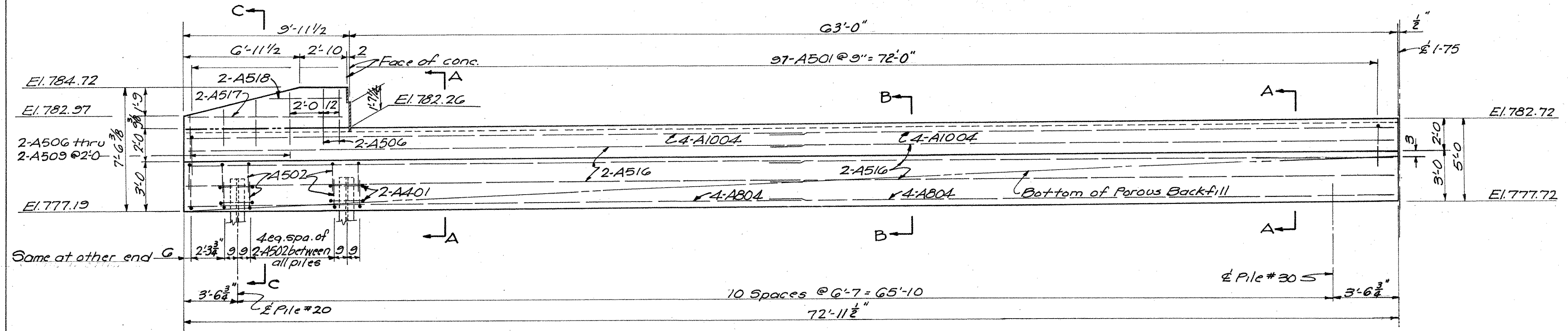
SHAW, LENZ & ASSOCIATES ENGINEERS 4/11
CINCINNATI OHIO

ABUTMENTS NO. 2
BRIDGE NO. MOT-75-1783
IR-75-OVER KEENAN AVENUE

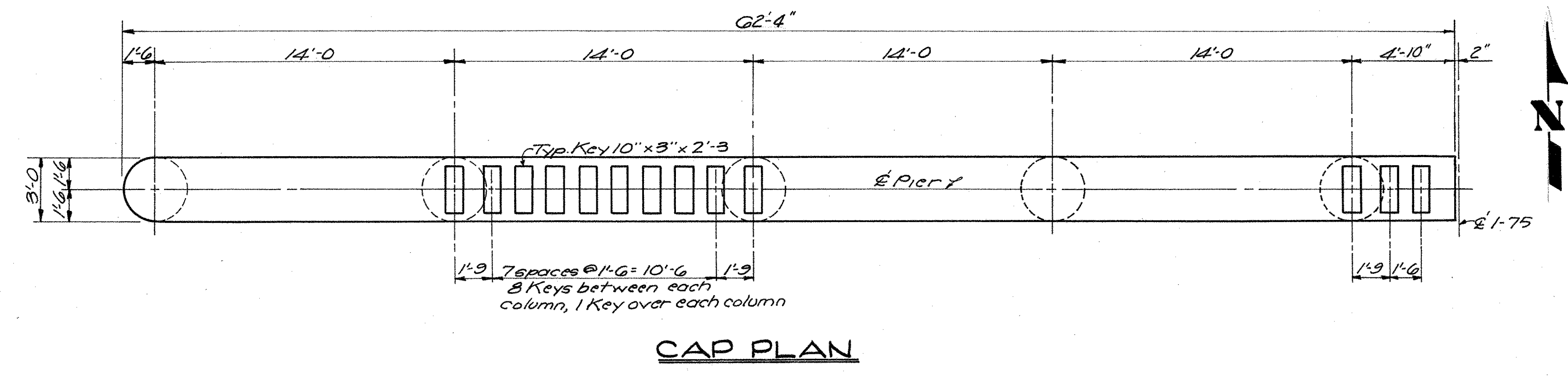
MONTEGOMERY COUNTY STA. 82+36.58 to STA. 83+42.58
DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
W.B.S. R.R.L. W.T.R. R.J.L. G-20-66



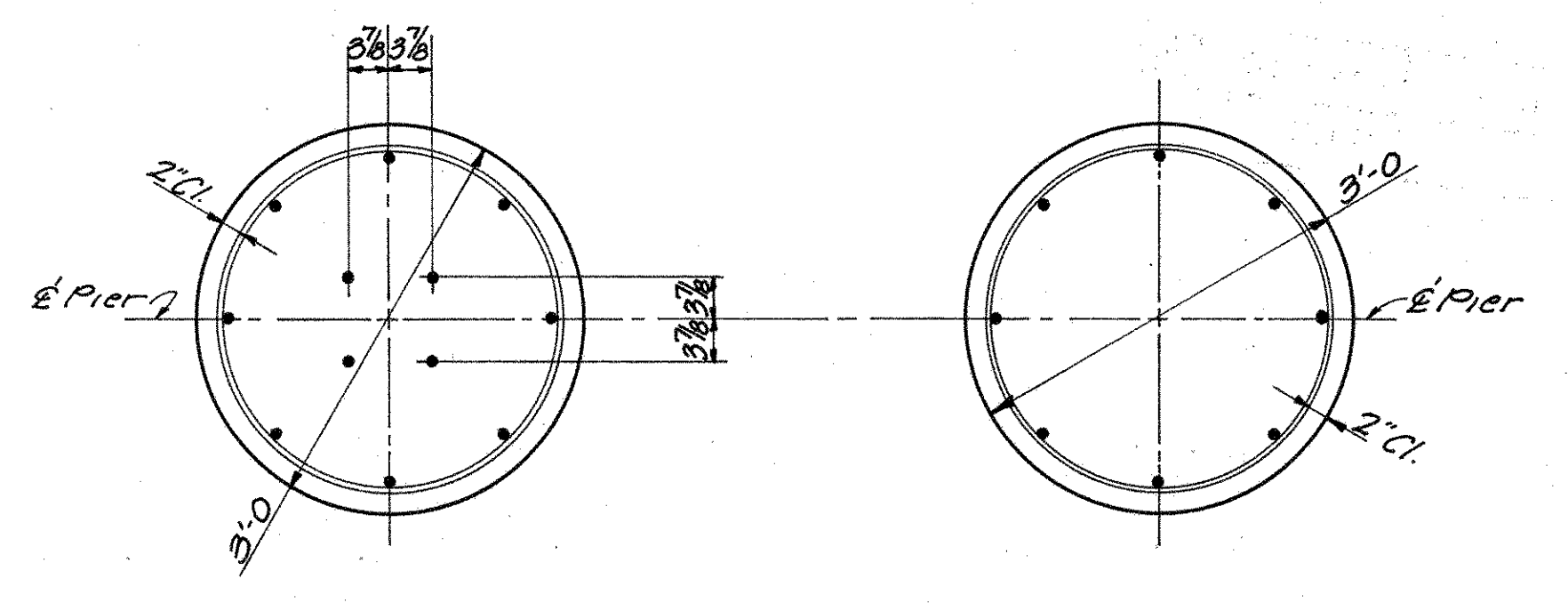
PLAN - LEFT BRIDGE



ELEVATION - LEFT BRIDGE

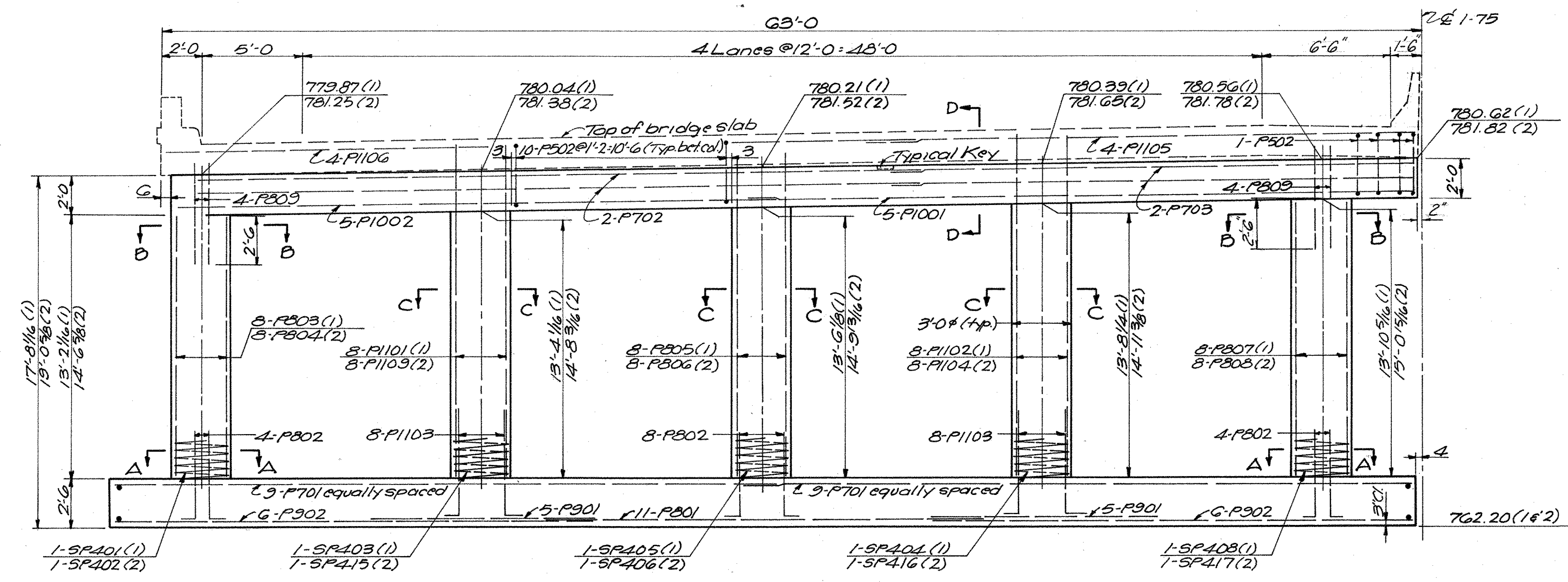


CAP PLAN

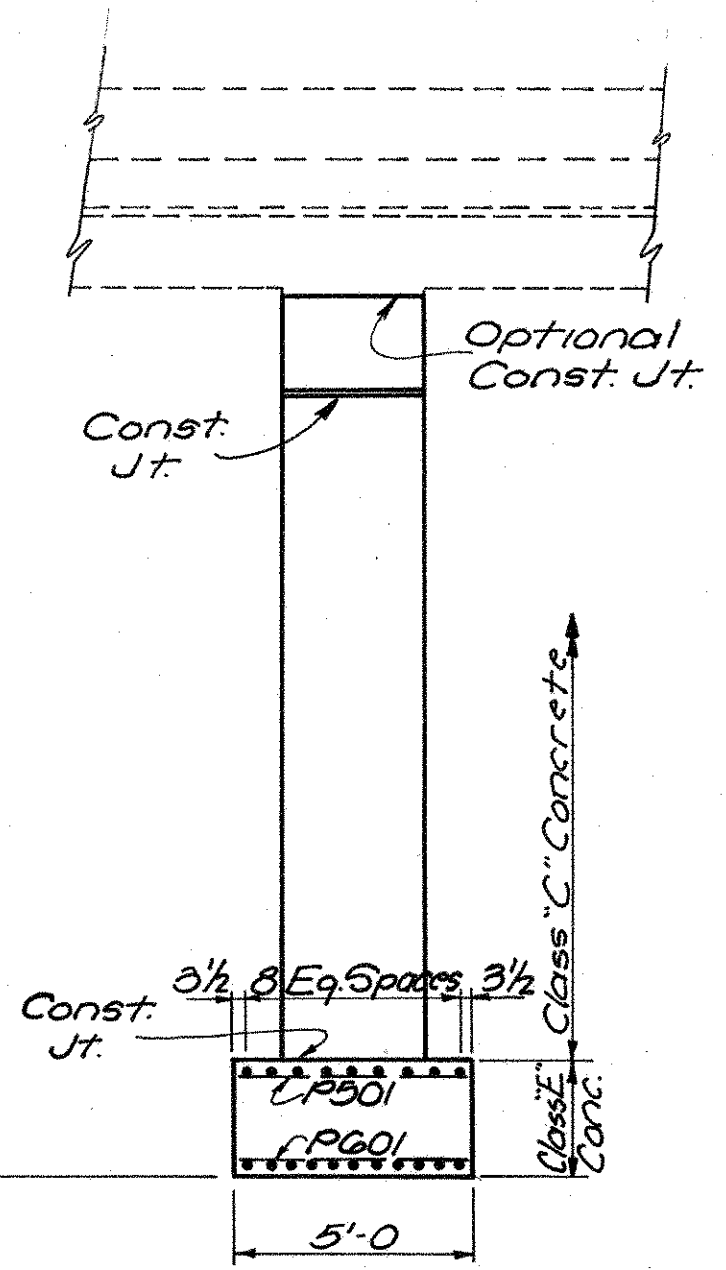


SECTION A-A & B-B

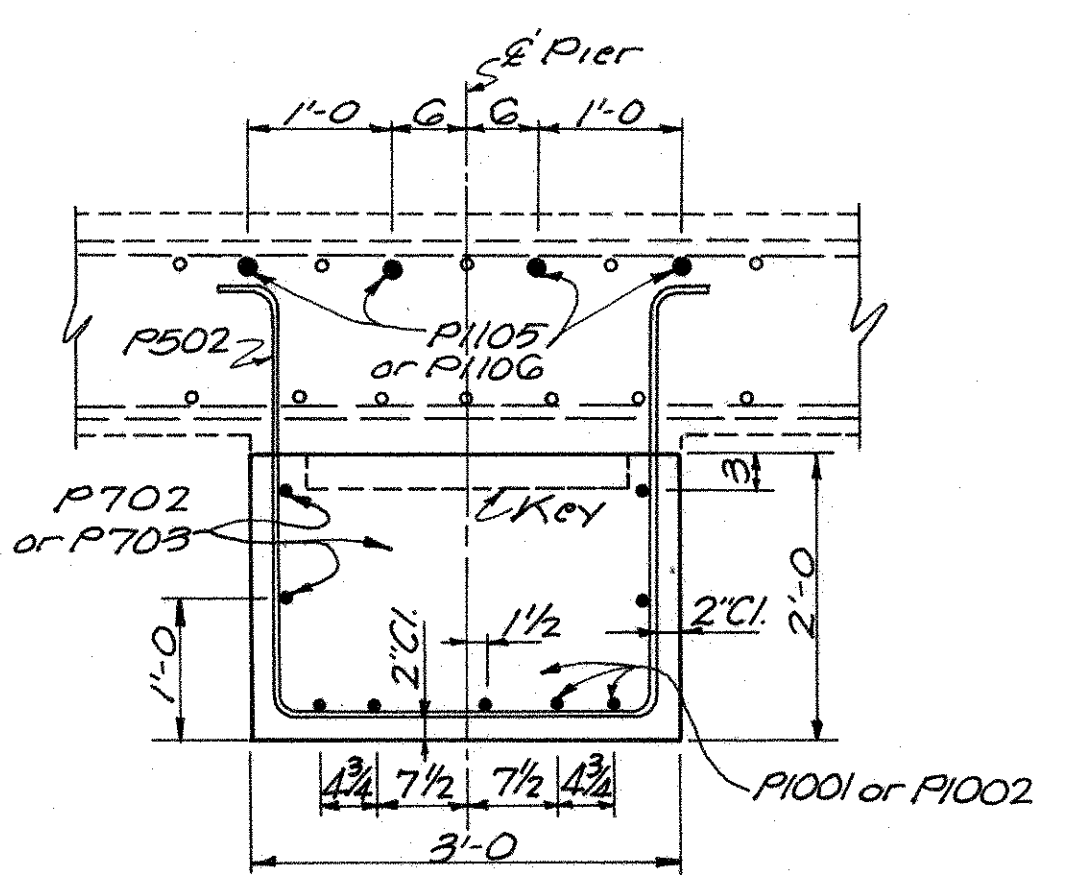
SECTION C-C



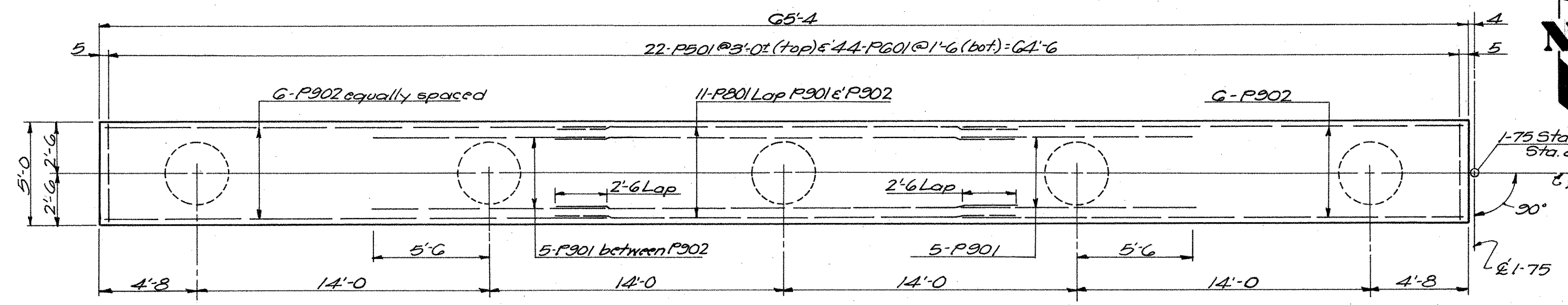
ELEVATION
(Looking North)



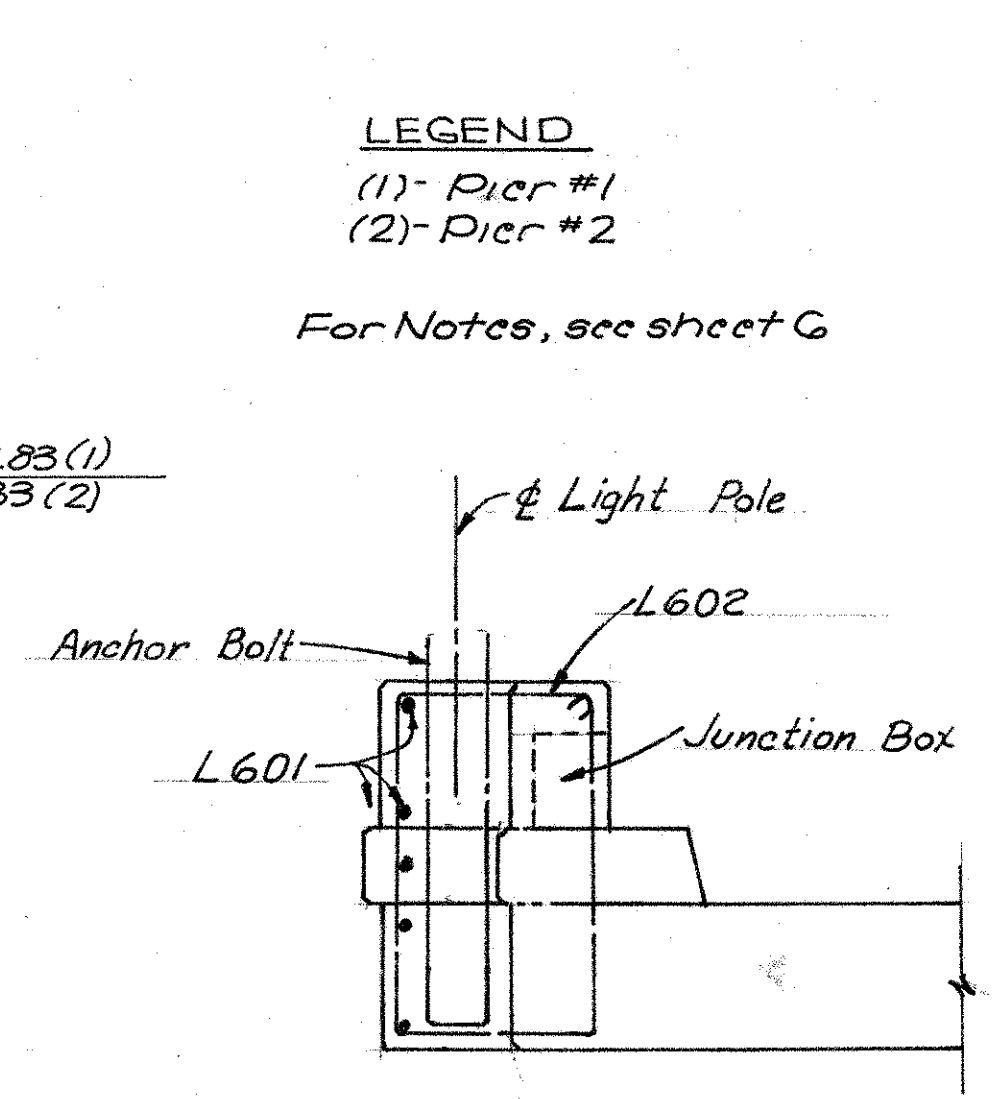
END VIEW



SECTION D-D

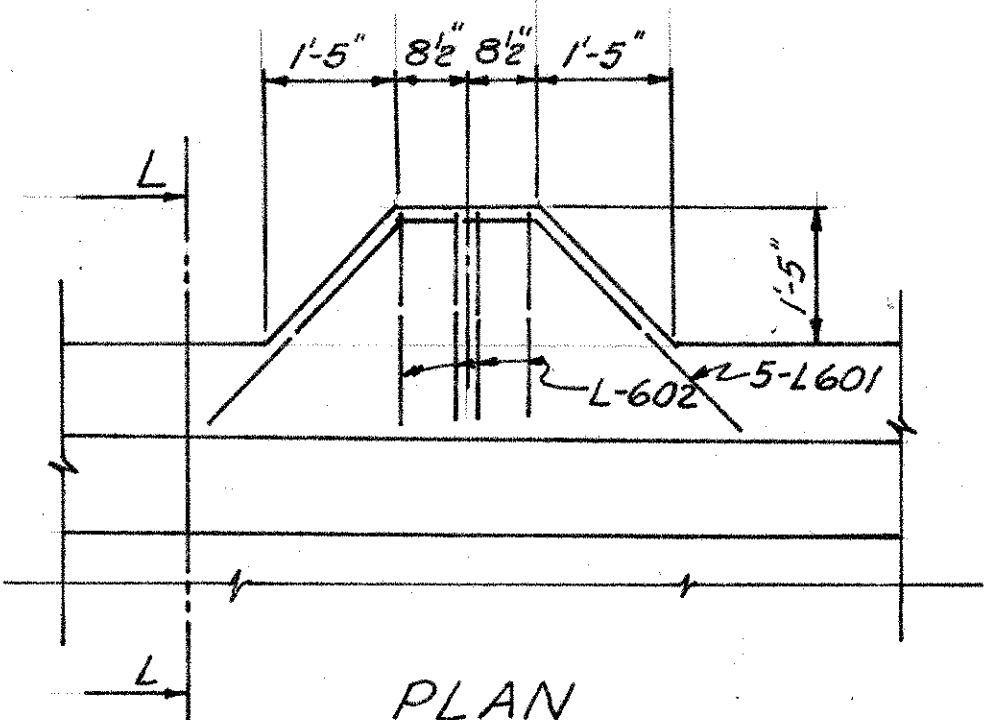


FOOTING PLAN
(Bottom Reinf. Steel Shown)



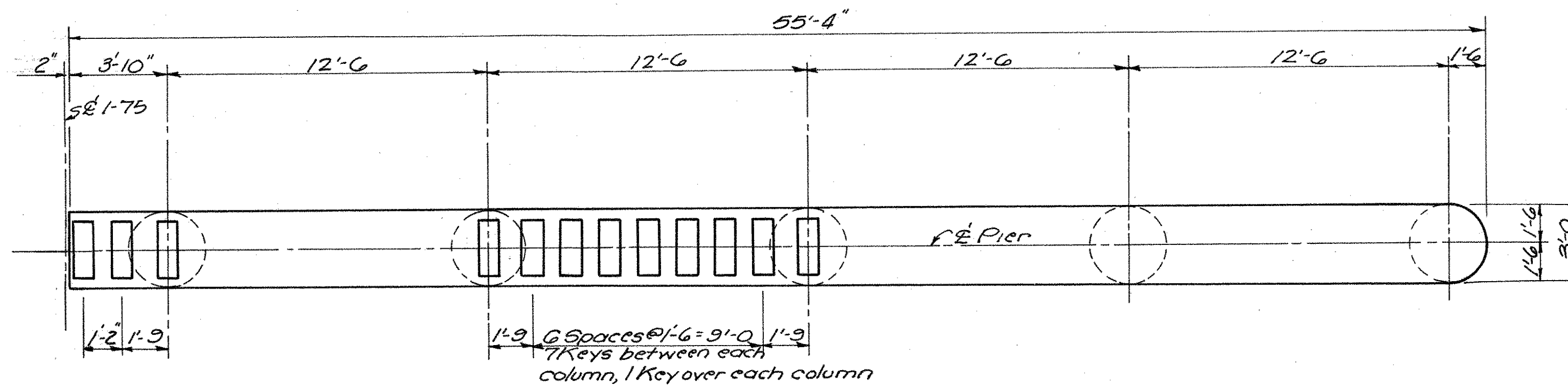
SECTION L-L

LEGEND
(1)- Pier #1
(2)- Pier #2
For Notes, see sheet G

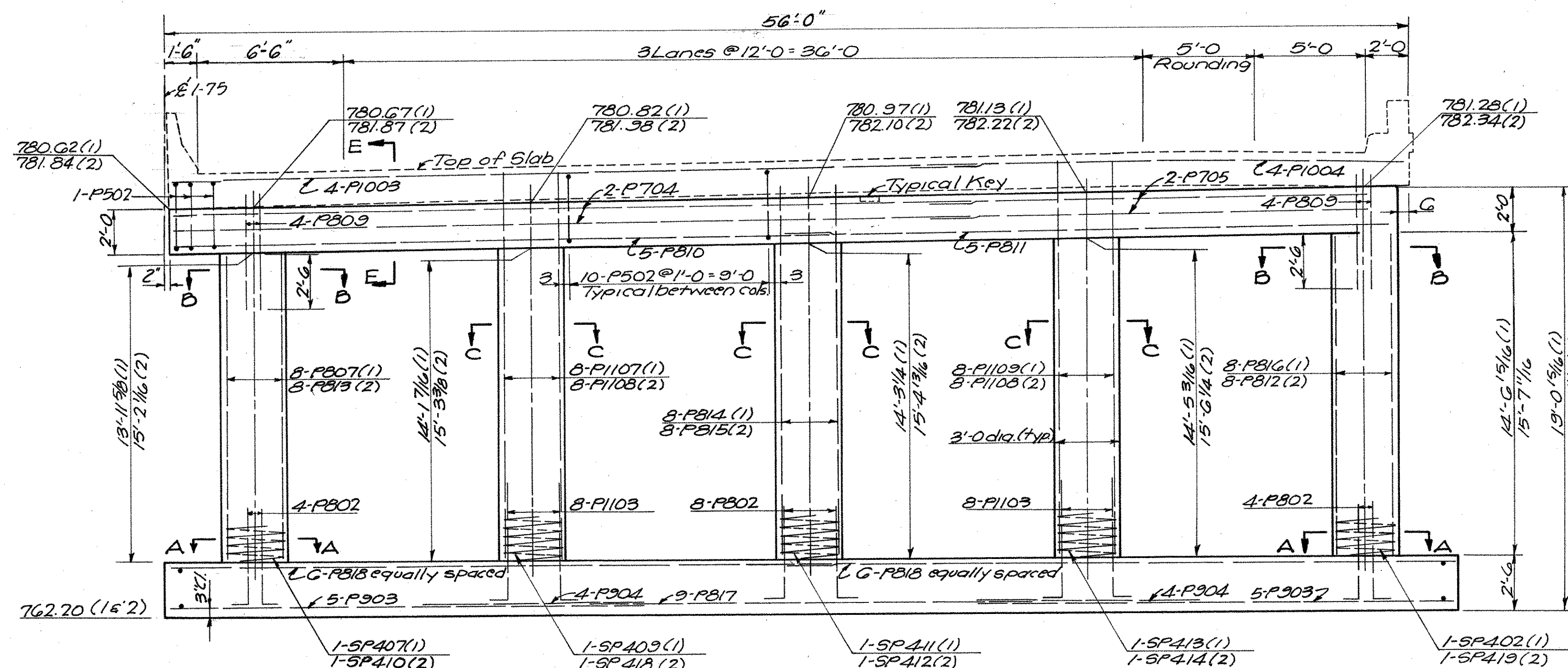


PLAN
LIGHT POLE SUPPORT
For details not shown see Lighting details.

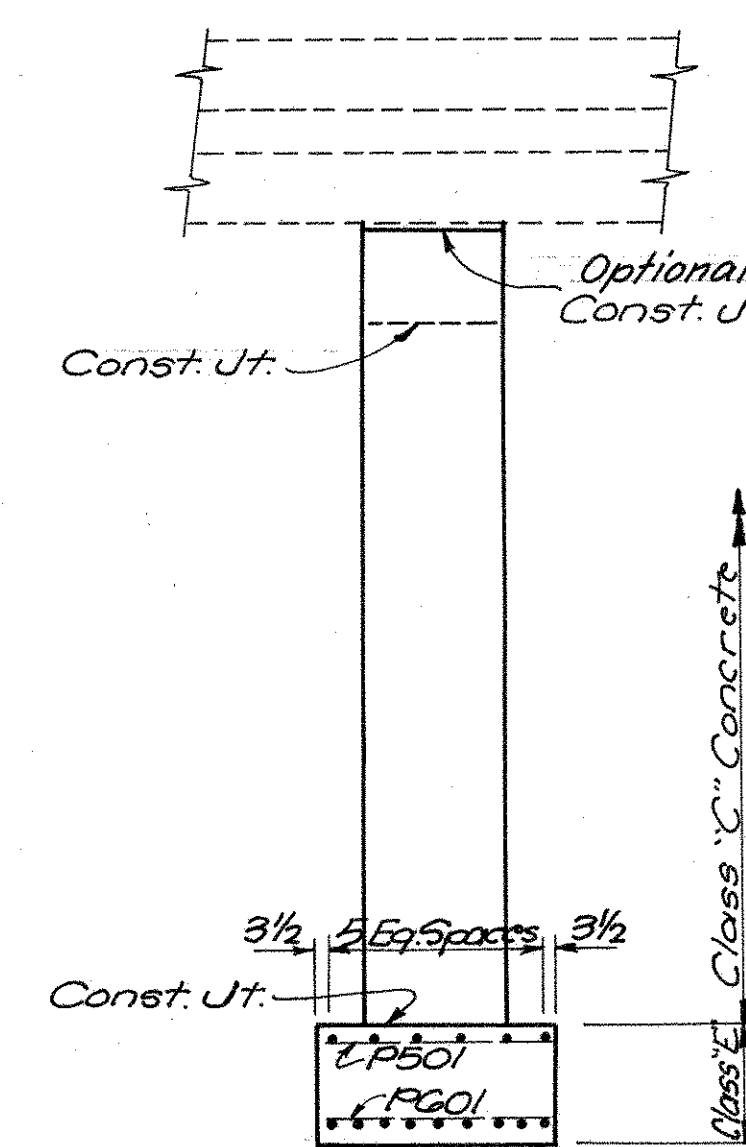
SHAW, LENZ & ASSOCIATES		5/11
ENGINEERS		
CINCINNATI		OHIO
PIERS NO. 1 & NO. 2		
LEFT BRIDGE		
BRIDGE NO. MOT-1783		
I-75 OVER KEENAN AVENUE		
MONTGOMERY COUNTY STA. 82+36.58 to STA. 83+42.58		
DESIGNED	DRAWN	TRACED
W.B.S.	R.R.L.	W.T.R.
		R.J.L.
		G-21-66



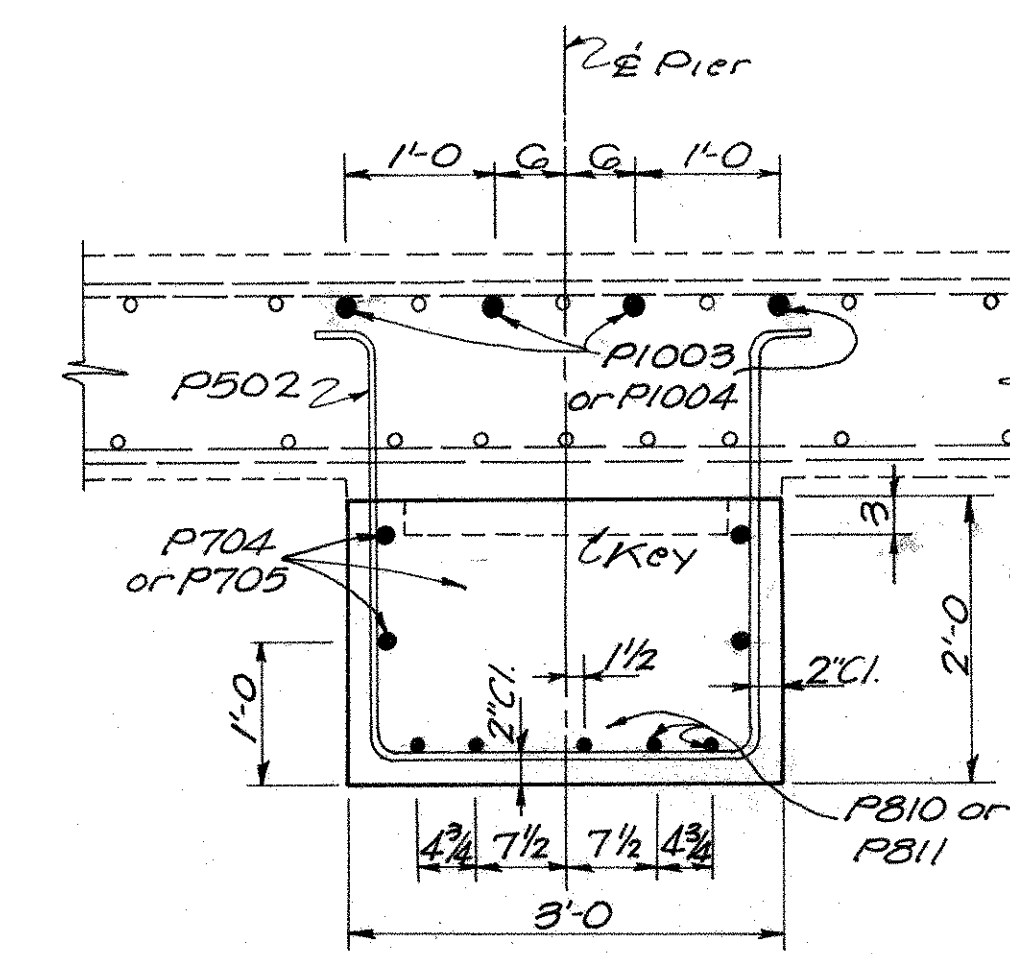
CAP PLAN



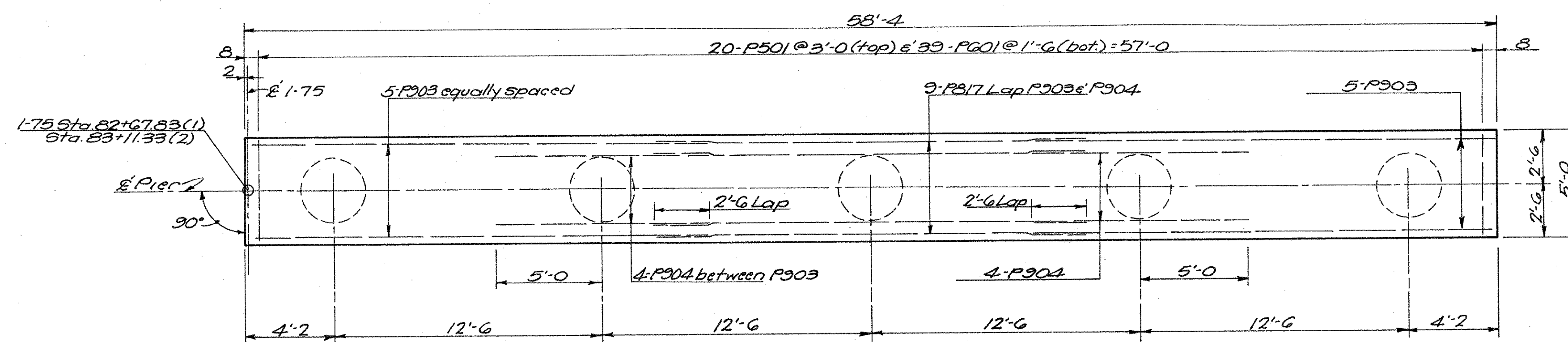
ELEVATION
(Looking North)



END VIEW



SECTION E-E



FOOTING PLAN
(Bottom Reinf. Steel Shown)

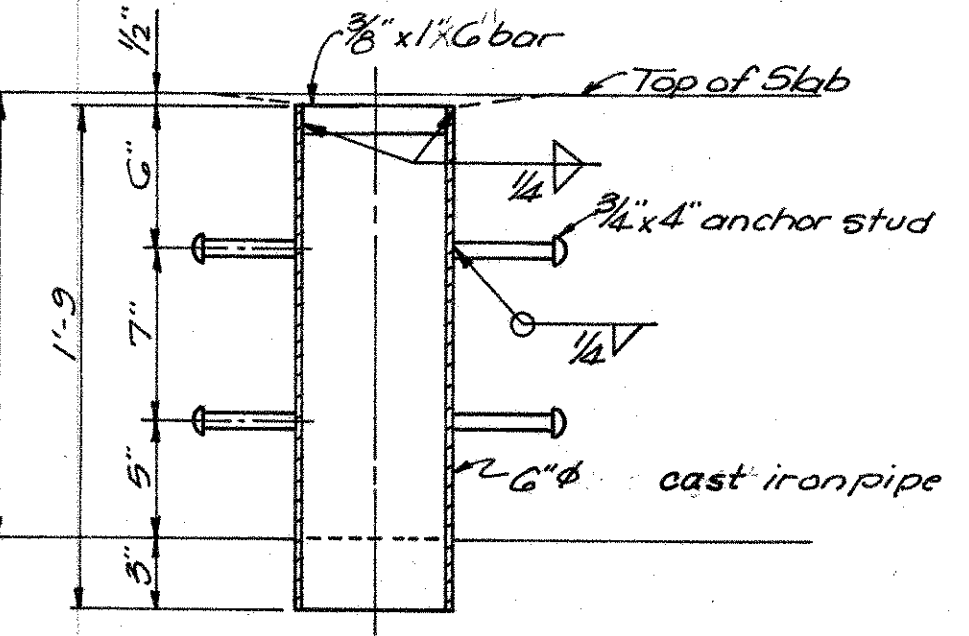
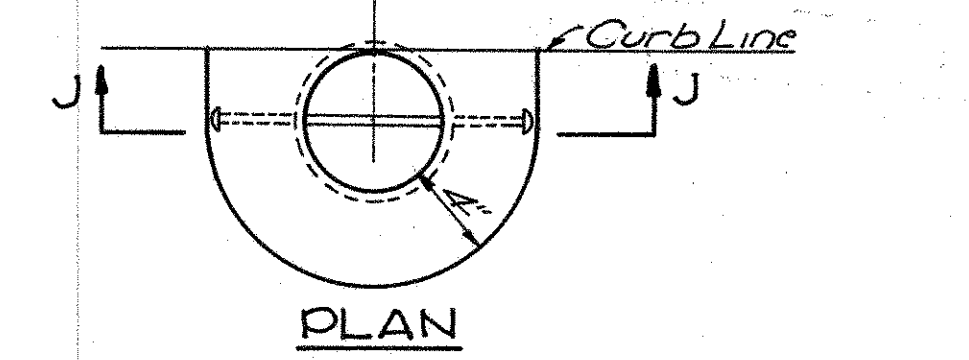
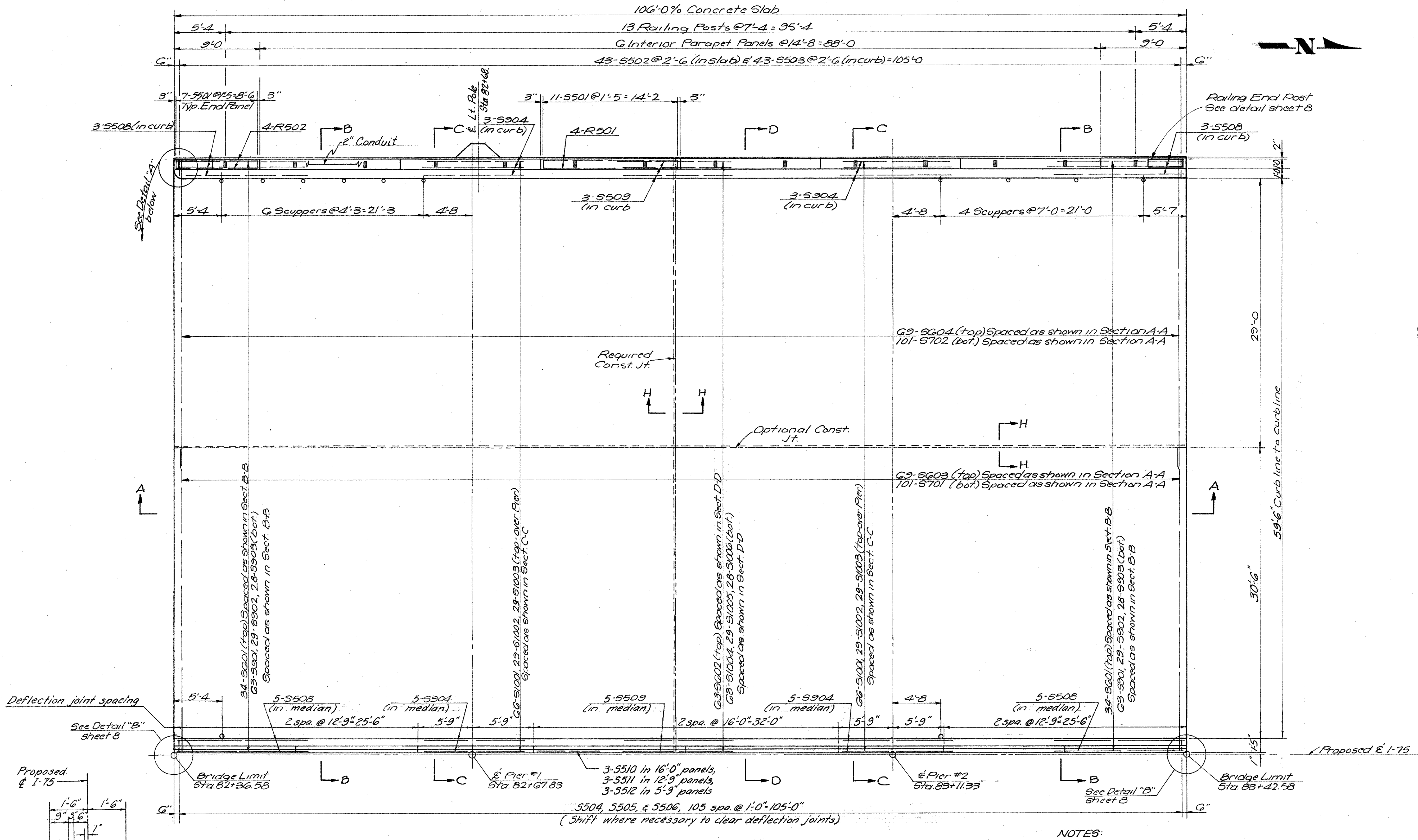
Top Longitudinal reinforcing shown in elevation view.

LEGEND
(1) - Pier #1
(2) - Pier #2

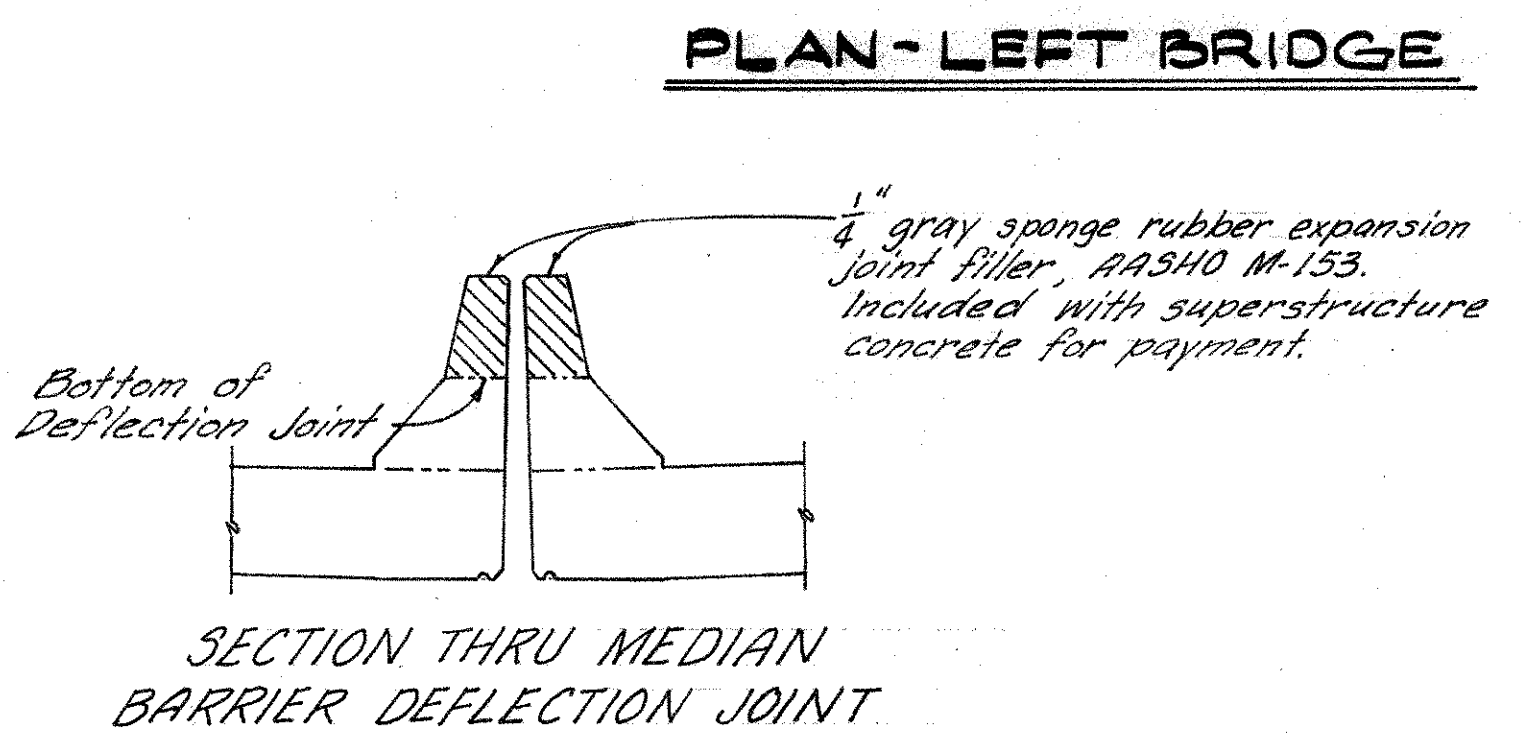
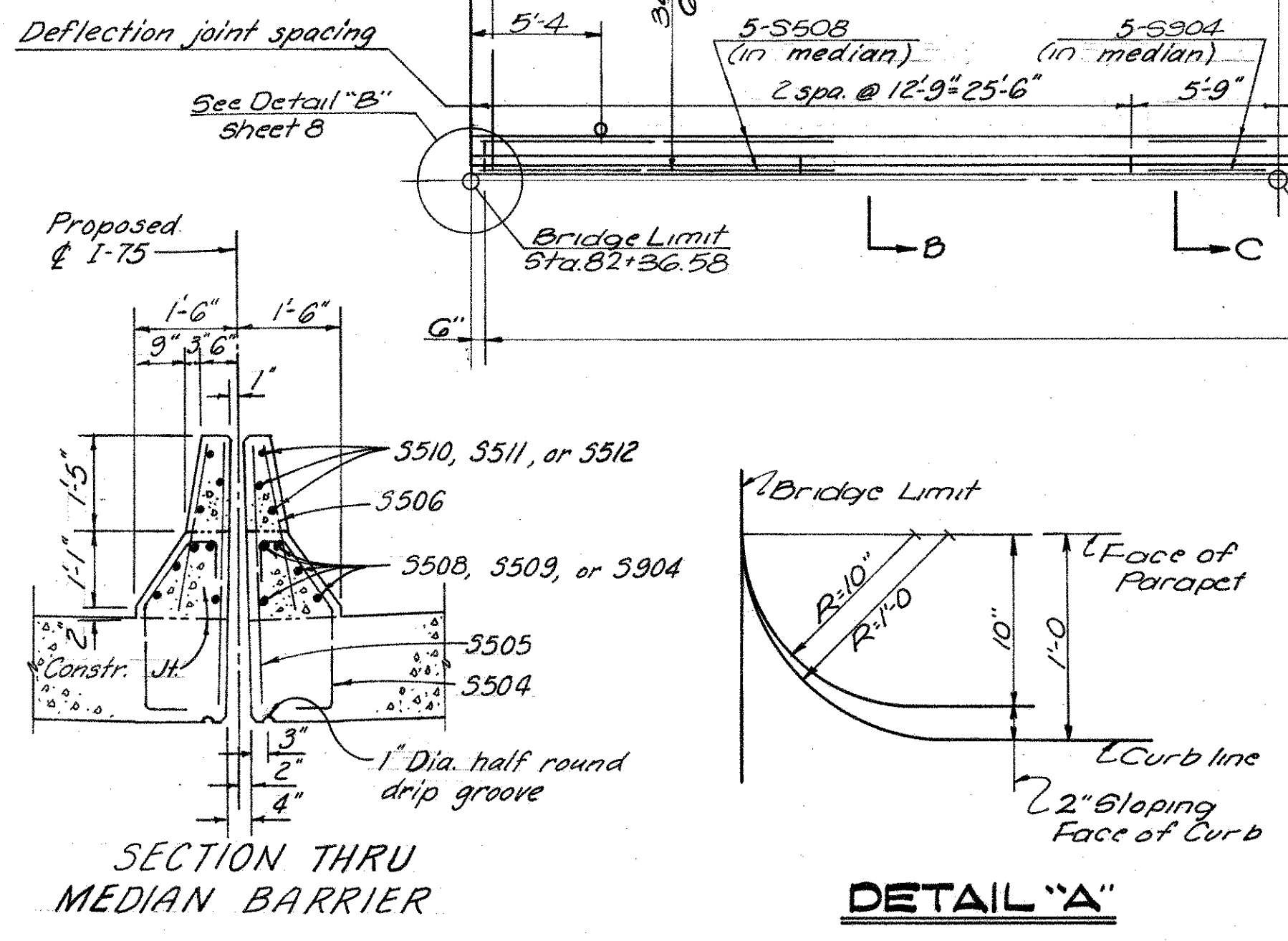
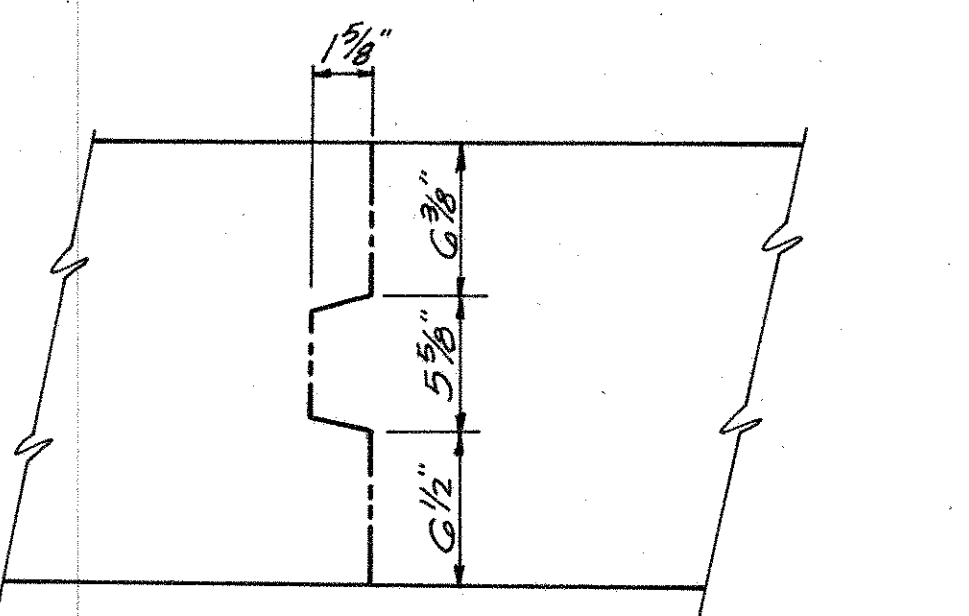
NOTES

1. For Sections A-A, B-B, & C-C, see Sheet 5.
2. The Pier Cap shall not be used to support falsework for the deck slab.
3. The construction joint, including shear keys, between the pier cap and slab is optional.

SHAW, LENZ & ASSOCIATES 6/11					
CINCINNATI			OHIO		
PIERS NO. 1 & NO. 2					
RIGHT BRIDGE					
BRIDGE NO. MOT-75-1783					
I-75 OVER KEENAN AVENUE					
MONTGOMERY COUNTY STA. 82+36.58 to STA. 83+42.58					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	R.R.L.	L.G.W.	R.J.L.	G-21-66	

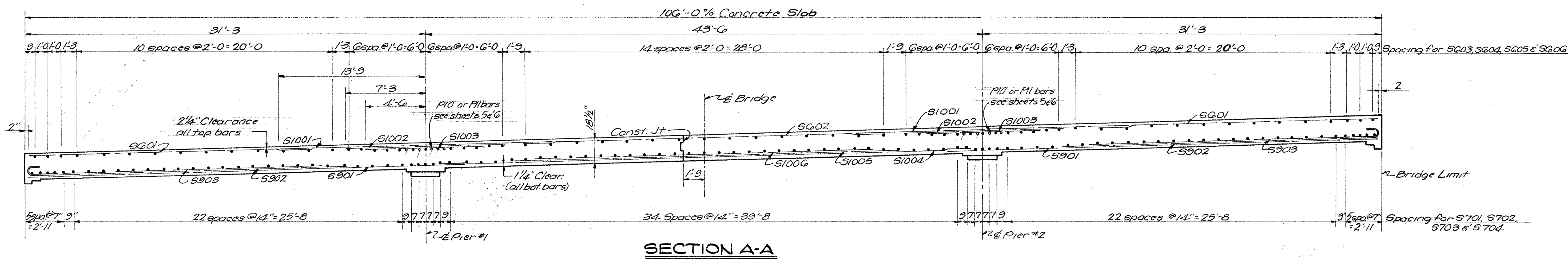


SCUPPER DETAIL

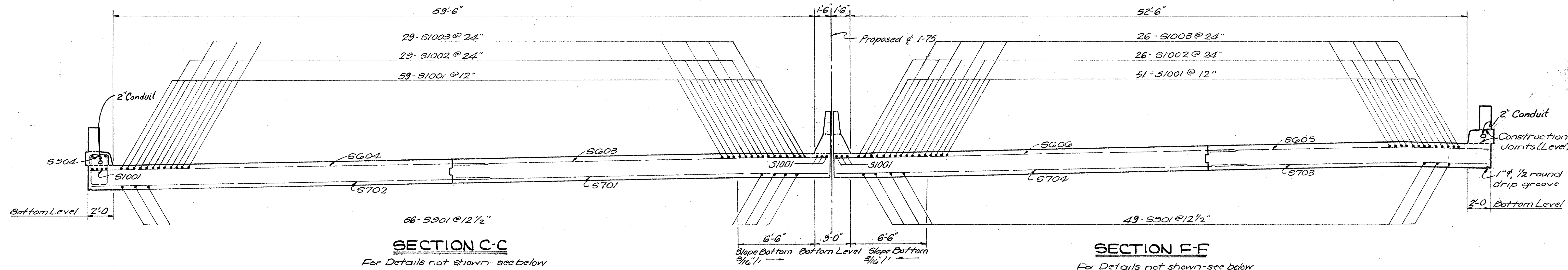


- NOTES:**
1. The concrete slab thickness of 18 1/2" includes a 1" monolithic wearing surface. The concrete quantity has been computed on this basis.
 2. See next sheet and Standard Dwg. BR-1-G5, sheet 1, for railing details.
 3. See sheet 9 for concrete slab sections.
 4. See sheet 10 for bridge slab and screed elevations. A camber for concrete dead load deflection has been included in the screed elevations. This is the amount of camber required before falsework is released. To obtain this, proper allowance shall be made for the deflection of falsework members.
 5. Curbs & median shall be placed after the shoring under the slab has been released sufficiently to permit the slab spans to attain full dead load deflection.
 6. For light pole support detail see sheet 375.

SHAW, LENZ & ASSOCIATES ENGINEERS					7/11
CINCINNATI					OHIO
SUPERSTRUCTURE SLAB LEFT BRIDGE					
BRIDGE NO. MOT-75-1753 1636					
IR-75 OVER KEENAN AVENUE					
MONTGOMERY COUNTY STA. 82+36.58 TO STA. 83+42.58					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	R.R.L.	W.T.R.	R.J.L.	G. 22-66	

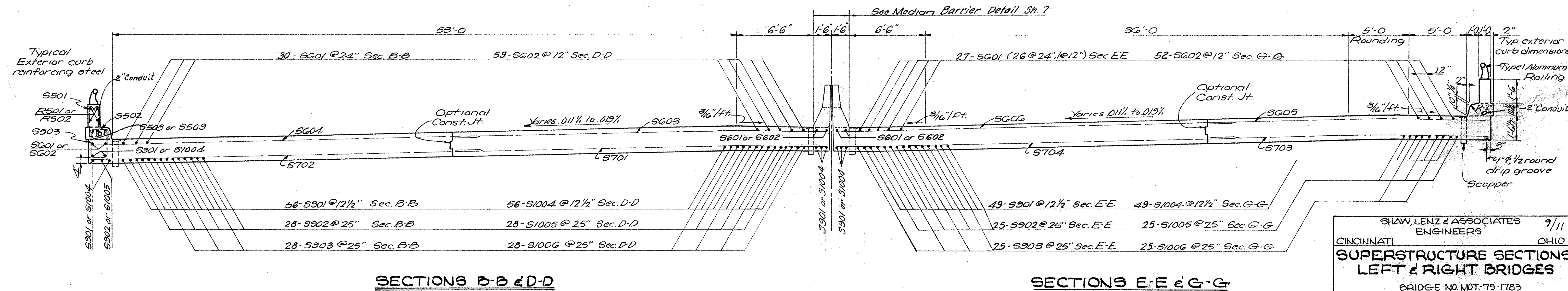


SECTION A-A



SECTION C-C
For Details not shown - see below

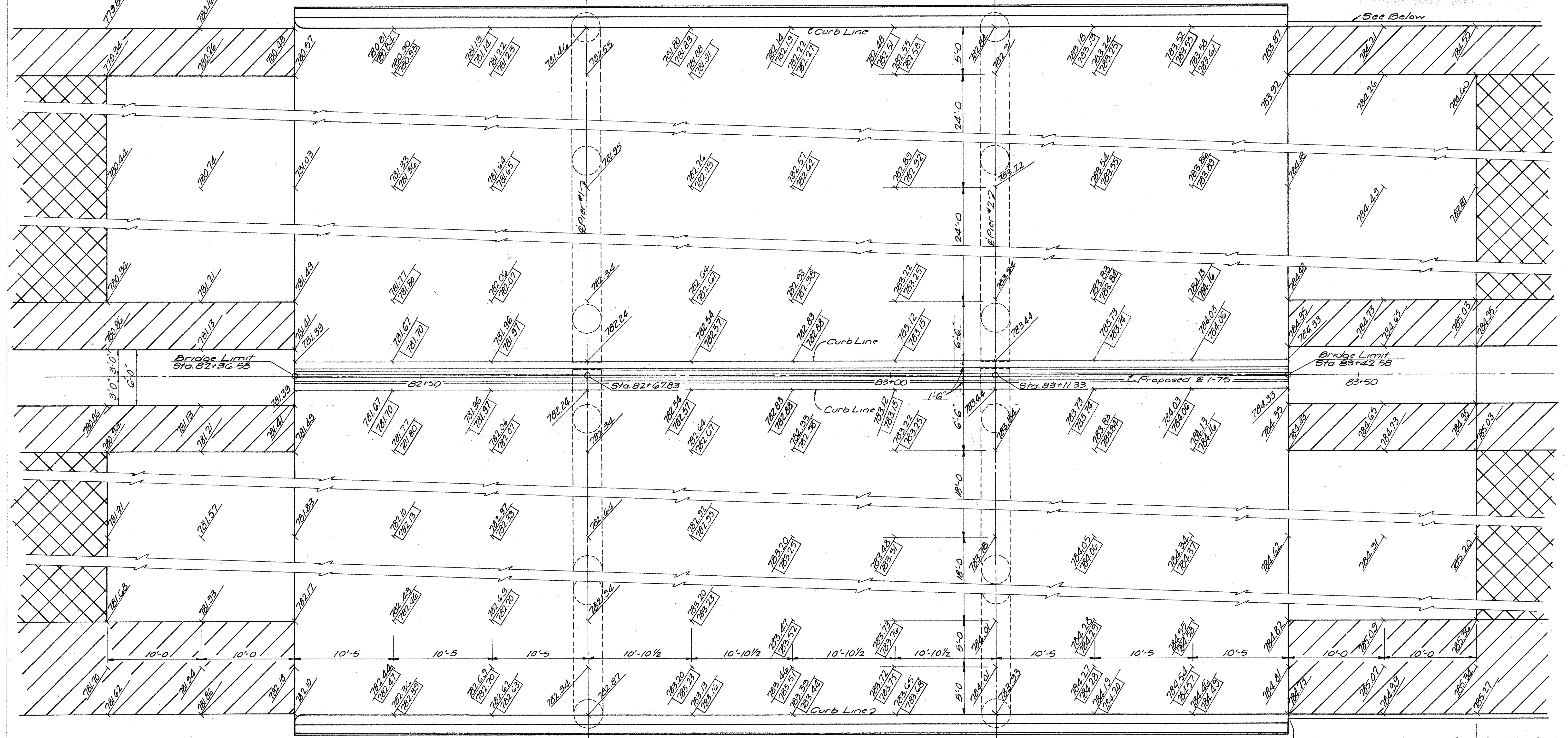
SECTION F-F
For Details not shown - see below



SECTIONS B-B & D-D

SECTIONS E-E & G-G

SHAW, LENZ & ASSOCIATES ENGINEERS 9/11
CINCINNATI OHIO
SUPERSTRUCTURE SECTIONS LEFT & RIGHT BRIDGES
BRIDGE NO. MOT-75-1783
IR-75 OVER KEENAN AVENUE
MONTGOMERY COUNTY STA. 82+36.58 to STA. 83+42.58
DESIGNED | DRAWN | TRACED | CHECKED | REVIEWED | REVISED
W.B.S. | W.B.S. | W.T.R. | R.J.L. | G-22-66



PLAN VIEW - SHOWING PAVEMENT & BRIDGE SLAB GRADES

20" Modified Type G Concrete Curb Std. Type G
 Vary face of curb from 10" at Concrete Curb
 bridge, to 6" in 20"

NOTE: Screed elevations allow for dead load deflections. To obtain this proper allowance shall be made for the deflection of falsework members.

LEGEND:

- Concrete Surface
- Shoulder Pavement
- Traveled Pavement
- Required Finished Grade
- Screed Elevation

SHAW, LENZ & ASSOCIATES 10/11
 ENGINEERS
 CINCINNATI OHIO

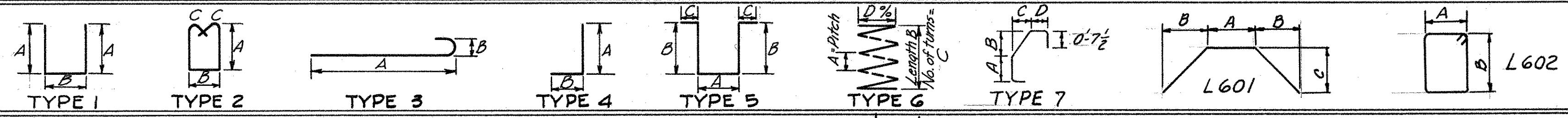
CONCRETE SLAB ELEVATIONS

BRIDGE NO. MOT-75-1783
 IR-75 OVER KEENAN AVENUE

MONTEGOMERY COUNTY STA. 82+36.58 TO STA. 83+42.58

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	W.B.S.	W.T.R.	R.J.L.	G-16-66	

REINFORCING STEEL BAR SCHEDULE



FED. RD. DIVISION	STATE	PROJECT
	OHIO	

579
 MOT. - 75-17.66

MARK	NO.	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
	LT.	RT.							
BRIDGE SLAB									
S1001	132	116	24'-7"	Str.				26,234	Over Piers, L&R
S1002	58	52	13'-4"	↑				6,311	"
S1003	58	52	8'-6"	↑				4,023	"
S1004	63	56	40'-0"	↓				20,482	L&R
S1005	29	26	27'-10"	↓				6,587	"
S1006	28	25	21'-0"	Str.				4,789	"
S901	126	112	35'-9"	Str.				28,929	L&R
S902	58	52	25'-1"	3	23'-10"	0'-11 1/4"		9,381	"
S903	56	50	21'-11"	3	20'-8"	0'-11 1/4"		7,899	"
S904	16	16	21'-0"	Str.				2,285	Median
S701	101		31'-8"	Str.				6,537	Left
S702	101		33'-1"	↓				6,830	"
S703		101	23'-10"	↑				4,920	Right
S704		101	34'-0"	Str.				7,019	"
S601	68	62	15'-3"	Str.				3,759	L&R
S602	63	56	25'-8"	↑				4,588	"
S603	69		31'-8"	↑				3,282	Left
S604	69		32'-9"	↑				3,394	"
S605	69		23'-10"	↓				2,470	Right
S606	69		33'-9"	Str.				3,489	"
S501	80	80	5'-7"	2	2'-2"	0'-8"	0'-7"	932	Curbs, L&R
S502	43	43	2'-7"	1	0'-8"	1'-6"		232	"
S503	43	43	4'-11"	1	1'-10"	1'-6"		441	Below Curbs, L&R
S504	106	106	3'-10"	7	1'-5"	0'-11 1/2"	0'-8"	848	Median
S505	106	106	3'-11"	Str.				866	Median
S506	106	106	2'-5"	Str.				534	Median
S508	16	16	22'-2"	Str.				740	Median
S509	8	8	25'-8"	Str.				428	Median
* R501	24	24	14'-4"	Str.					Int. Parapets, L&R
* R502	8	8	8'-8"	Str.					End Parapets, L&R
* R503	6	6							End Posts, L&R
* R504	4	4							"
ABUTMENTS									
A1001	8		35'-11"	Str.				1,236	Abut. #1, Left
A1002	8		32'-1"	↓				1,104	Abut. #1, Right
A1003	8		34'-0"	↓				1,170	Abut. #2, Right
A1004	8		37'-11"	Str.				1,305	Abut. #2, Left
A801	8		35'-7"	Str.				760	Abut. #1, Left
A802	8		31'-9"	↓				678	Abut. #1, Right
A803	8		33'-8"	↓				719	Abut. #2, Right
A804	8		37'-7"	Str.				803	Abut. #2, Left
A501	188	168	7'-11"	1	3'-3"	1'-8"		2,940	Abut. #1 & #2, L&R
A502	206	186	6'-7"	1	2'-1"	2'-8"		2,692	"
A503	12		35'-2"	Str.				440	Abut. #1, Left
A504	2		5'-7"	↑				12	"
A505	2		2'-9"	↑				6	"
A506	8	6	3'-6"	↑				51	Abut. #1 & #2, L&R
A507	4	4	3'-0"	↑				25	"
A508	4	4	2'-6"	↑				21	"
A509	4	4	2'-0"	↑				17	"
A510	12		31'-2"	↓				392	Abut. #1, Right
A511	2		5'-0"	↓				10	"
A512	2		2'-5"	↓				5	"
A513	12		33'-4"	↓				417	Abut. #2, Right
A514	2		9'-0"	Str.				19	"

MARK	NO.	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
	LT.	RT.							
A515		2	4'-10"	Str.				10	Abut. #2, Right
A516	12		37'-2"	↓				465	Abut. #2, Left
A517	2		9'-8"	↓				20	"
A518	2		5'-3"	Str.				11	"
A401	84	76	5'-5"	1	1'-11"	1'-9"		579	Abuts. #1 & #2, L&R
PIERS									
P1101	8		16'-8"	Str.				708	Col. Pier #1-L
P1102	8		17'-0"	Str.				723	Col. Piers #1-L
P1103	32	32	7'-2"	4	5'-10"	1'-7"		2,437	Pier Ftgs.
P1104	8		18'-3"	Str.				776	Col. Pier #2-L
P1105	8		27'-6"	↑				1,169	Slab, over Piers #1-L & #2-L
P1106	8		38'-7"	↑				1,640	"
P1107		8	17'-5"	↓				740	Col. Pier #1-R
P1108		16	18'-7"	↓				1,580	Col. Pier #2-R
P1109	8	8	17'-9"	Str.				1,509	Col. Piers #1-R & #2-L
P1001	10		34'-3"	Str.				1,474	Cap. Piers #1-L & #2-L
P1002	10		31'-0"	↑				1,334	"
P1003		8	36'-6"	↓				1,256	Slab, over Piers #1-R & #2-R
P1004		8	22'-3"	Str.				766	"
P901	20		11'-0"	Str.				748	Ftgs. Piers #1-L & #2-L
P902	24		23'-11"	↓				1,952	"
P903		20	21'-6"	↑				1,462	Ftgs. Piers #1-R & #2-R
P904		16	10'-0"	Str.				544	"
P801	22		22'-0"	Str.				1,292	Ftgs. Piers #1-L & #2-L
P802	32	32	5'-4"	4	4'-9"	9"		911	Pier Ftgs.
P803	8		13'-2"	Str.				281	Col. Pier #1-L
P804	8		14'-6"	↑				310	Col. Pier #2-L
P805	8		16'-0"	↑				342	Col. Pier #1-L
P806	8		17'-3"	↑				368	Col. Pier #2-L
P807	8	8	13'-10"	↑				591	Col. Pier #1-L & #2-R
P808	8		15'-1"	↑				322	Col. Pier #2-L
P809	16	16	5'-0"	↑				427	Pier Cols.
P810		10	30'-2"	↑				805	Cap. Piers #1-R & #2-R
P811		10	27'-7"	↑				736	"
P812		8	15'-7"	↑				333	Col. Pier #2-R
P813		8	15'-2"	↑				324	Col. Pier #2-R
P814		8	16'-7"	↑				354	Col. Pier #1-R
P815		8	17'-10"	↑				381	Col. Pier #2-R
P816		8	14'-7"	↑				312	Col. Piers #1-R
P817		18	20'-0"	↓				961	Ftgs. Piers #1-R & #2-R
P818		24	30'-3"	Str.				1,938	"
P701	36		33'-8"	Str.				2,477	Ftgs. Piers #1-L & #2-L
P702	8		35'-5"	↑				579	Cap. Piers #1-L & #2-L
P703	8		27'-5"	↑				448	"
P704		8	33'-4"	↓				545	Cap. Piers #1-R & #2-R
P705		8	22'-6"	Str.				368	"
P601	88	78	4'-8"	Str.				1,164	Pier Ftgs.
P501	44	40	4'-8"	Str.				409	Pier Ftgs.
P502	86	86	8'-11"	5	2'-8"	3'-0"	0'-5"	1,600	Pier Caps
SUPERSTRUCTURE (Continued)									
S510	6	6	15'-8"	Str.				196	Median
S511	12	12	12'-5"	Str.				311	"
S512	12	12	5'-5"	Str.				136	"
L601	5		7'-0"	Bt.	1'-3"	2'-1"	2'-1"	55	Light Pole
L602	4		11'-8"	Bt.	2'-1"	3'-6"		70	"

MARK	NO.	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
	LT.	RT.							
SPA01	1		13'-2"	G	4 1/2"	13'-2"	38	2'-8"	245
SPA02	1	1	14'-7"	↑	↑	14'-7"	42	↑	542
SPA03	1		13'-4"	↑	↑	13'-4"	39	↑	252
SPA04	1		13'-8"	↑	↑	13'-8"	39	↑	252
SPA05	1		13'-6"	↑	↑	13'-6"	39	↑	252
SPA06	1		14'-9"	↑	↑	14'-9"	40	↑	257
SPA07	1	1	13'-11"	↑	↑	13'-11"	40	↑	258
SPA08	1		13'-10"	↑	↑	13'-10"	40	↑	258
SPA09	1		14'-1"	↑	↑	14'-1"	41	↑	265
SPA10	1		15'-2"	↑	↑	15'-2"	43	↑	278
SPA11	1		14'-3"	↑	↑	14'-3"	41	↑	265
SPA12	1		15'-4"	↑	↑	15'-4"	44	↑	284
SPA13	1		14'-5"	↑	↑	14'-5"	41	↑	265
SPA14	1		15'-6"	↑	↑	15'-6"	44	↑	284
SPA15	1		14'-8"	↑	↑	14'-8"	42	↑	271
SPA16	1		14'-11"	↑	↑	14'-11"	43	↑	278
SPA17	1		15'-1"	↑	↑	15'-1"	43	↑	278
SPA18	1		15'-3"	↑	↑	15'-3"	44	↑	284
SPA19	1		15'-7"	G	4 1/2"	15'-7"	45	2'-8"	291
REPLACEMENT BARS									
RE101	1		8'-7"	Str.					
RE1001	2	2	8'-3"	↑					
RE901	2	1	7'-10"	↑					
RE801	1		7'-6"	↑					
RE701	1	1	7'-3"	↑					
RE601	1	1	6'-11"	↑					
RE501	1		6'-7"	↓					
RE401	1		6'-3"	Str.					

NOTES:

- All dimensions are out to out of bar.
- Radius dimensions are to outside of bar.
- The length of bent bars is measured along centerline.
- Bar size is indicated in the bar mark. The first digit where three digits are used and the first two digits where four are used indicates the bar size.
- Four steel channel, tee or angle spacers, weighing approximately 0.68

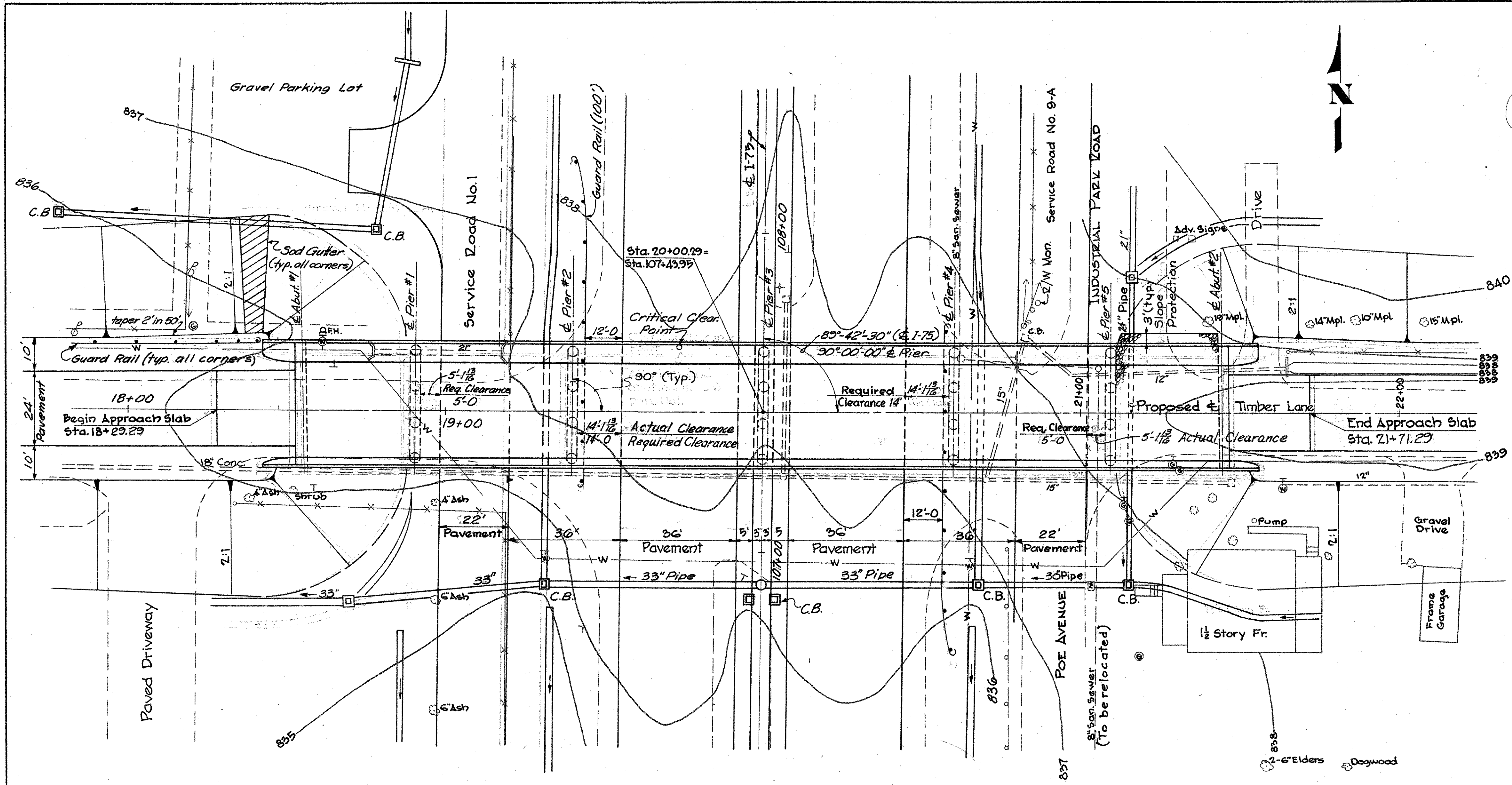
Red 370

PROPOSED STRUCTURE
 Type: Continuous steel beam with reinf. conc. deck and substructure.
 Spans: 35'-0", 49'-0", 59'-9", 49'-0", 35'-0".
 Roadway: 5'-0" f/c curbs with one 2'-0" safety curb and one 6'-0" sidewalk.
 Load Frequency: C.F. = 130 (57)
 Skew: 0°
 Wearing Surface: 1" monolithic conc.
 Approach Slabs: AS-1-67 (25' long)
 Alignment: Tangent

AVERAGE DAILY TRAFFIC
 A.D.T. (1975).....1520
 TYPE "B" (Estimated).....152

PEDESTRIAN TRAFFIC
 Two hour morning count (7:00 to 9:00).....12
 1 1/2 hour afternoon count (2:30 to 4:00).....48
 Total count (April, 1965).....60 *
 Estimated daily traffic (1965).....80
 Estimated daily traffic (1975).....135
 * Includes a count of 24 from Coronette Ave. which is to be closed.

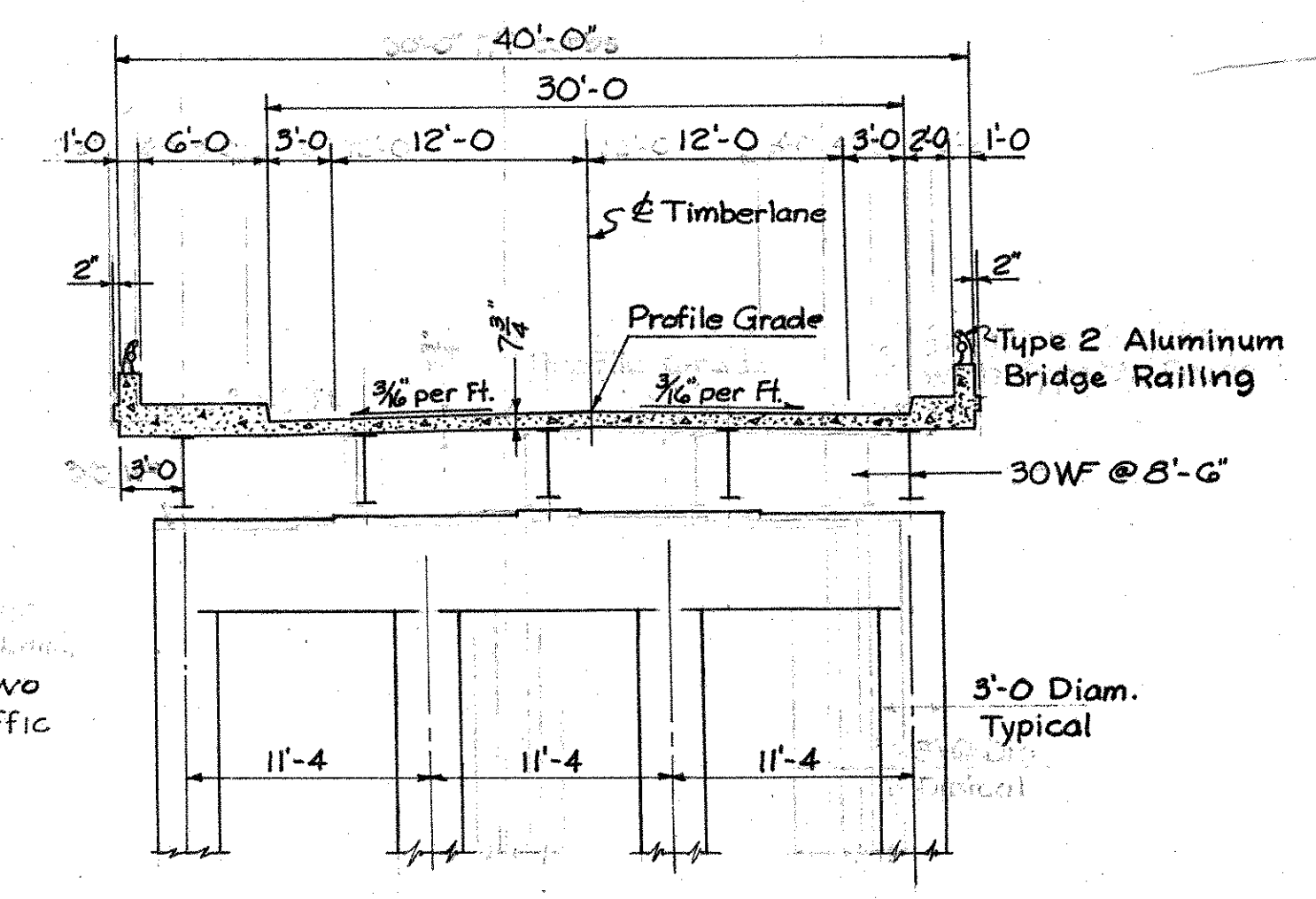
Bench Mark Elev. 833.410
 2" dia cut in W. side ditch inlet structure
 108' Rt. of Sta. 103+82



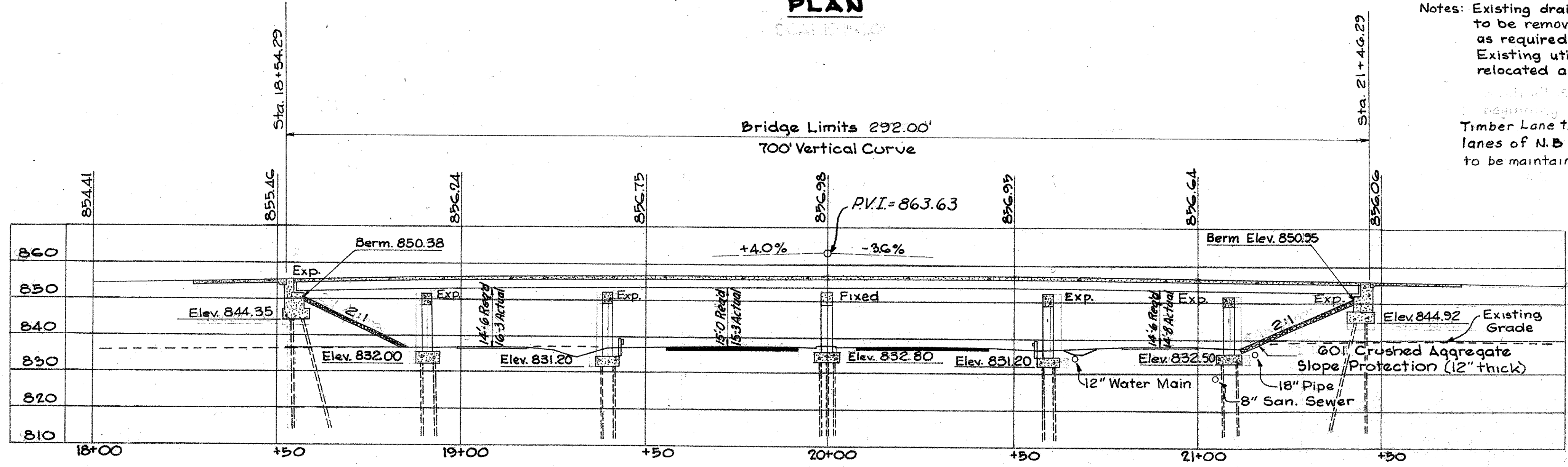
PLAN

Notes: Existing drainage structures to be removed or abandoned as required.
 Existing utilities to be relocated as required.

Timber Lane traffic to be detoured. Two lanes of N.B. & two lanes of S.B. traffic to be maintained on I-75



SECTION - BRIDGE DECK



SECTION ALONG & TIMBER LANE

Note: All piles are 12" cast-in-place concrete piles.

Estimated pay lengths:
 Piers = 20'
 Abuts. = 30'

Note: Piling size and type to be determined after soil borings are made.
 All piers and abutments are parallel and 90° to & of Timberlane.

SHAW, LENZ & ASSOCIATES 1/10
 ENGINEERS
 CINCINNATI OHIO

SITE PLAN
 BRIDGE NO. MOT. - 75-1831
 TIMBER LANE OVER I-75
 MONTGOMERY CO I-75 STA. 107+25.78
 Scale: As Shown

PRESENT TOPO	PROPOSED WORK
SURVEYOR B.H.S.	DRAWN C.M.R.
DESIGNED E.R.B.	CHECKED C.M.R.
DATE 8-30-66	REVIEWED W.B.S.

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	ABUTS.	PIERS	SUPER-STRUCTURE	GENERAL
503	595	Cu.Yds.	Excavation for Structures, unclassified	183	352		
505	Lump	Lump Sum	First Test Pile				Lump
507	2360	Lin.Ft.	12" ϕ Cast-in-Place Reinforced Concrete Piles	720	1640		
509	154,201	Lbs.	Reinforcing Steel	94,98	48,984	95,719	
511	381	Cu.Yds.	Class "C" Concrete- Superstructure			381	
511	140	Cu.Yds.	Class "C" Concrete- Piers above Footings		140		
511	81	Cu.Yds.	Class "C" Concrete- Abutments above Footings	81			
511	176	Cu.Yds.	Class "C" Concrete- Footings	66	110		
513	200,724	Lbs.	Structural Steel			200,724	
514	200,724	Lbs.	Field Painting of structural steel			200,724	
517	625	Lin.Ft.	Bridge Railing, Type 2	47		578	
518	28	Cu.Yds.	Porous Backfill	28			
518	12	Each	Scuppers, including supports			12	
518	65	Lin.Ft.	6" Perforated Helical CMP including specials 707.01	65			
518	50	Lin.Ft.	6" Non-perforated Helical CMP, 707.01	50			
601	378	Sq.Yds.	Crushed Aggregate Slope Protection				378
808	381	Units	Water-reducing, Set-retarding admixture			381	

GENERAL NOTES

REFERENCE shall be made to the following:

Standard Drawings: DR-1-65 sheet 2 revised 11-24-65
 RB-1-55 revised 2-2-59
 SD-1-65 sheets 1, 2 & 3 dated 11-8-65
 AS-1-67 Dated 1-11-68
 Supplemental Specifications: 808 dated 1-1-69
 811 dated 1-1-69
 836 dated 6-11-69

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57 together with current revisions thereof.

DESIGN LOADING: CF = 130 (57)

CONCRETE: Class "C" basic unit stress = 1333 p.s.i. for superstructure
 Class "C" basic unit stress = 1133 p.s.i. for substructure

STRUCTURAL STEEL: ASTM A-36 - basic unit stress = 20,000 p.s.i.

SCUPPERS including support angles shall be galvanized in accordance with 711.02

REINFORCING STEEL: ASTM, A615, A616, A617, Deformed Intermediate or Hard. Basic unit stress = 20,000 p.s.i.
 Spiral reinforcement shall be plain A306 or A499.

PILES shall be driven to a minimum bearing capacity of 25 tons per pile for the abutments and 35 tons per pile for the piers.

MACHINE FINISH: The concrete deck shall be finished by the use of a finishing machine.

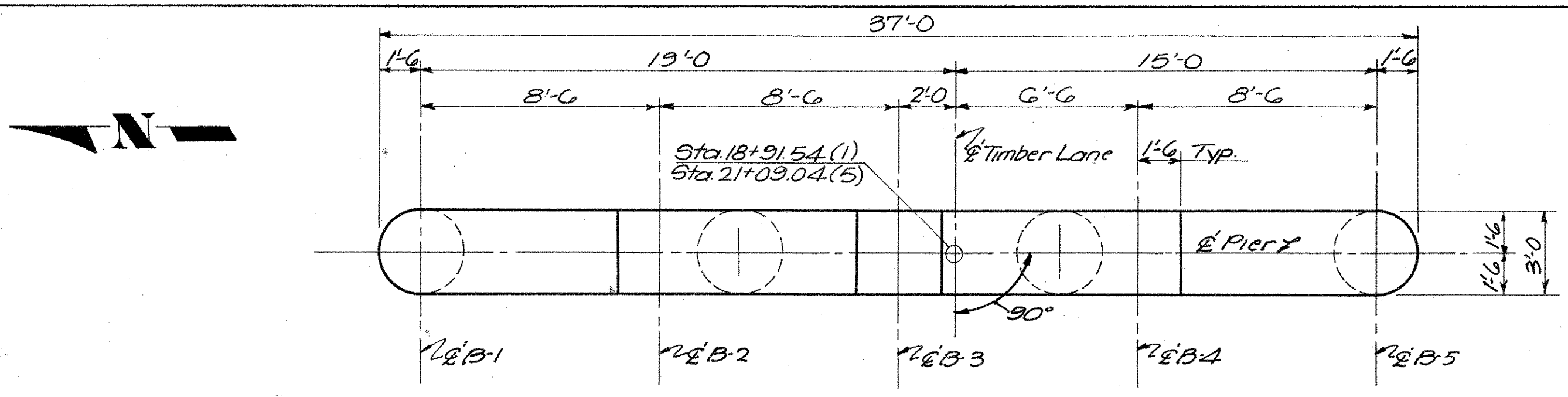
UTILITY LINES: All expenses involved in relocating the affected utility lines shall be borne by the owners. The Contractors and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either is held to a minimum.

EXCAVATION QUANTITY includes the removal of fill materials required for construction of the abutments.

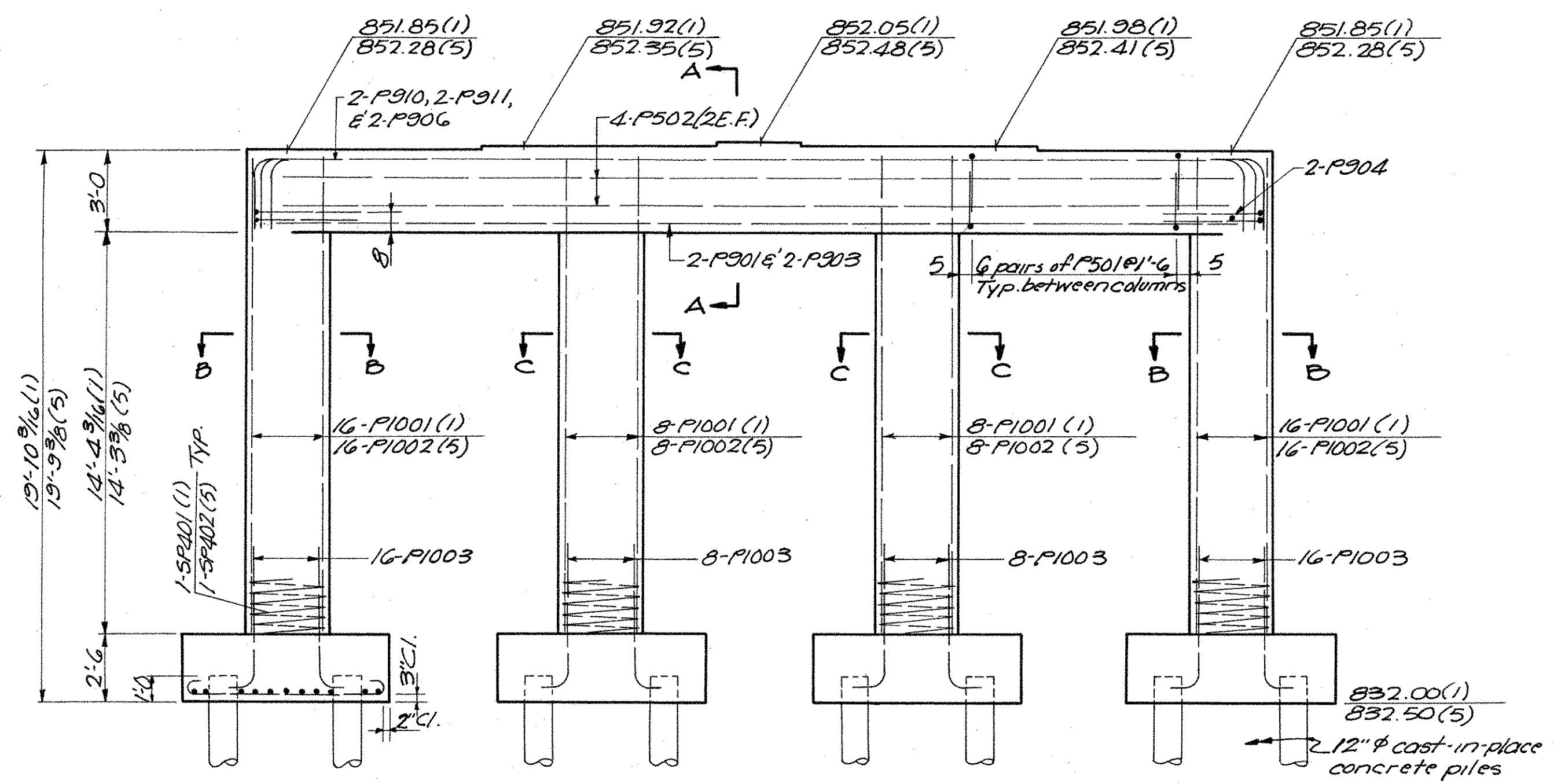
JOINT SEALER: Item 828 joint sealer including bond breaker, shown in Section A-A of Std. Dwg. SD-1-65, sheet No. 1 shall be omitted.

WELDS: Non-stress carrying welds are indicated thus: 

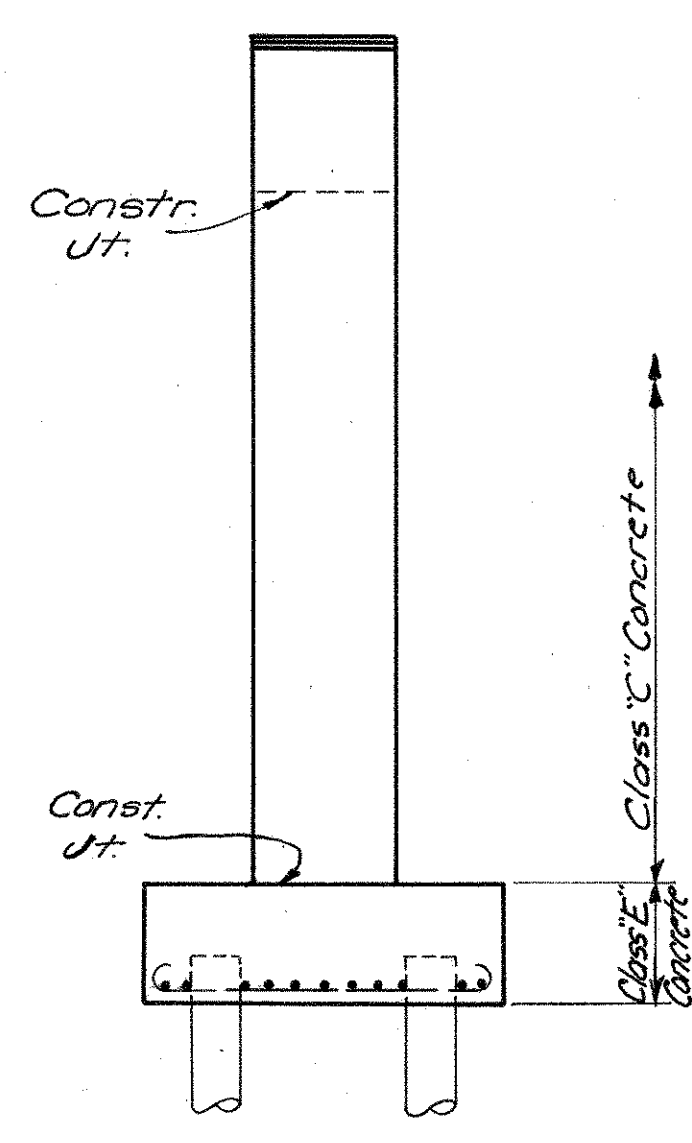
SHAW, LENZ & ASSOCIATES 2/10 ENGINEERS					
CINCINNATI			OHIO		
QUANTITIES & GENERAL NOTES					
BRIDGE NO. MOT-75-1831 TIMBER LANE OVER I-75 I-75					
MONTGOMERY CO.			STA. 107+25.78		
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
R.R.L.	R.R.L.	W.T.R.	R.J.L. 8-30-66		



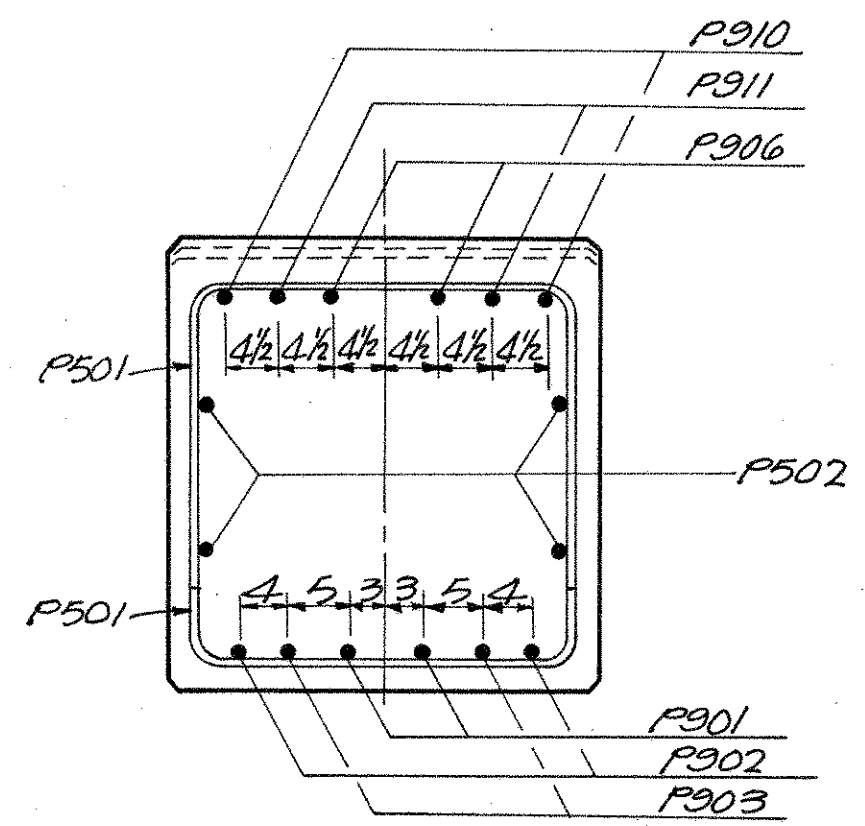
CAP PLAN



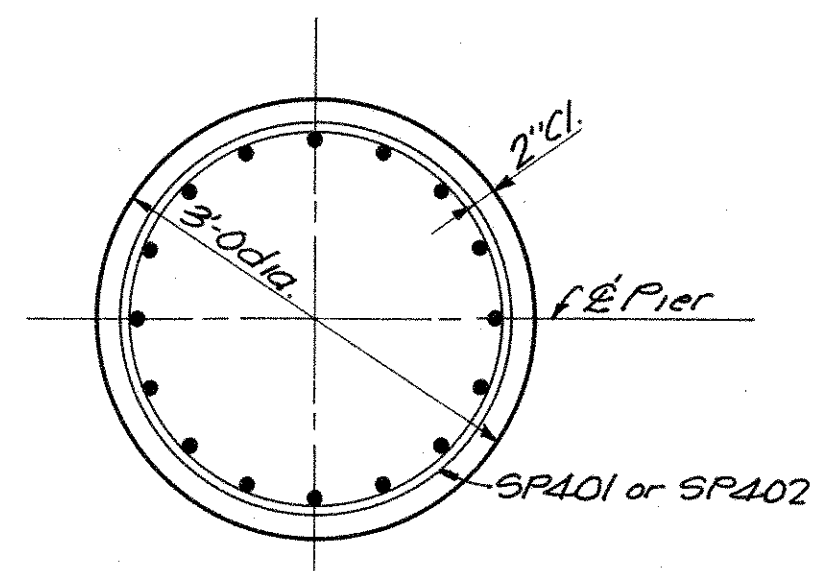
ELEVATION



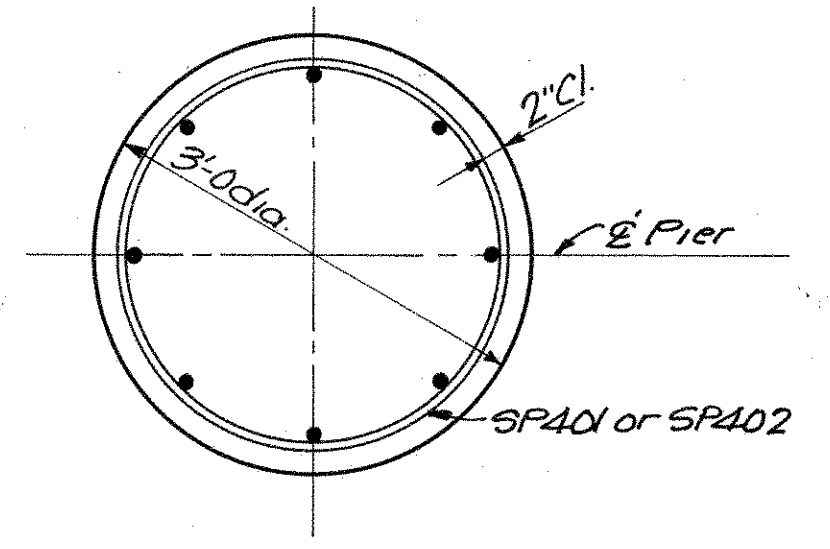
END VIEW



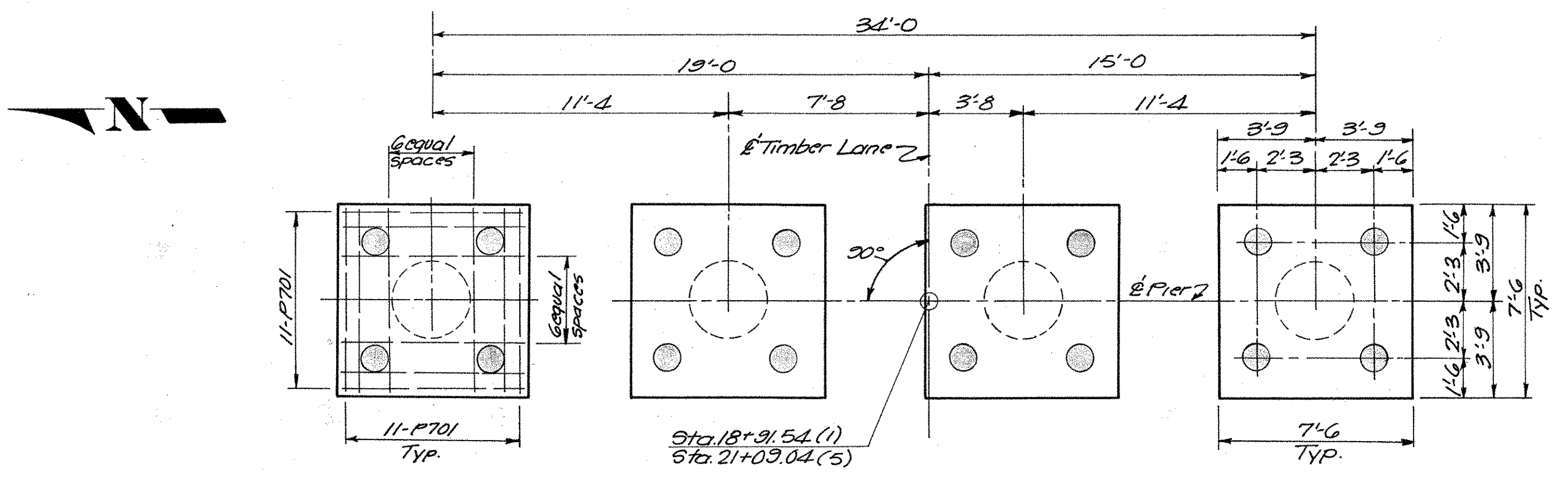
SECTION A-A



SECTION B-B

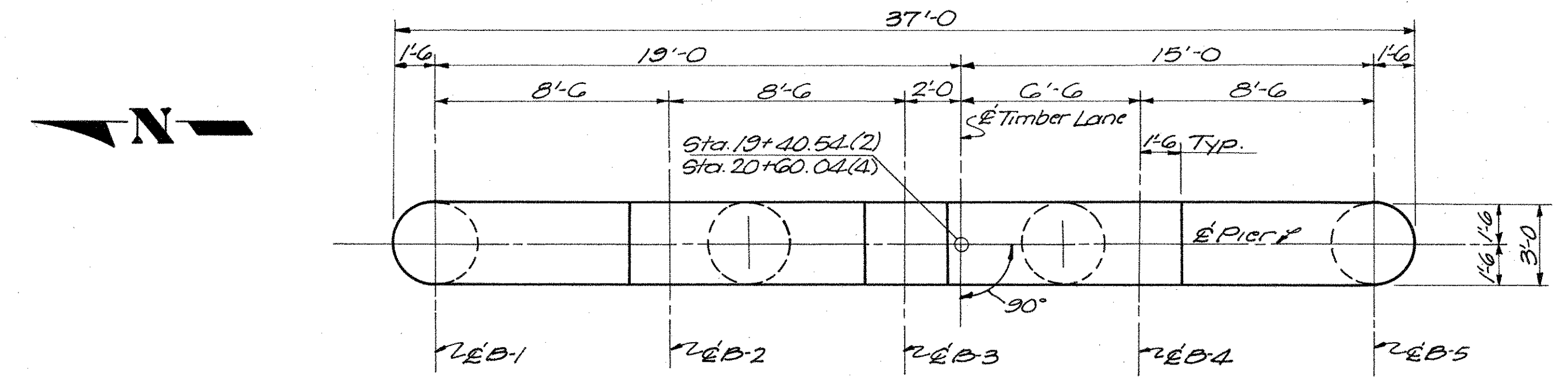


SECTION C-C

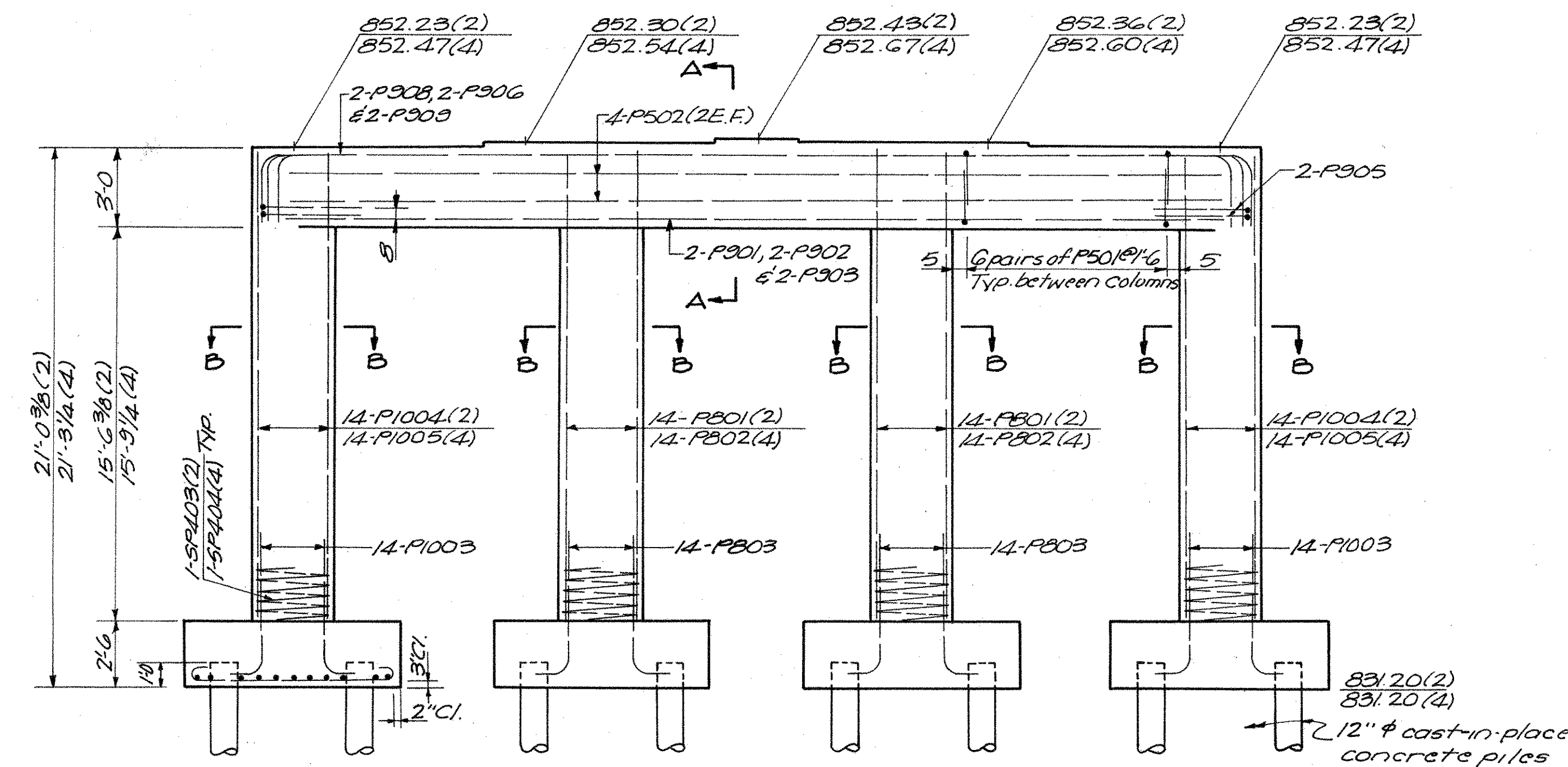


FOOTING PLAN

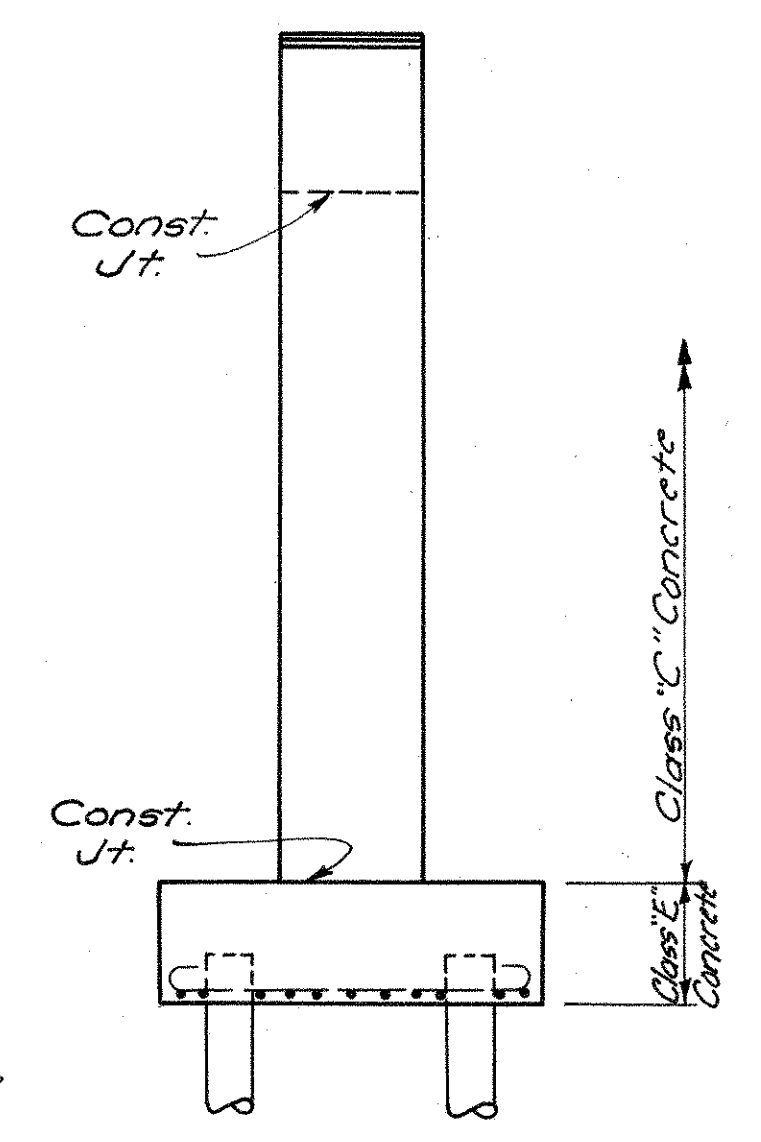
SHAW, LENZ & ASSOCIATES 5/10 ENGINEERS OHIO			
PIERS NO. 1 & NO. 5			
BRIDGE NO. MOT-75-1831 TIMBER LANE OVER I-75			
I-75 STA. 107+25.78			
DESIGNED	DRAWN	TRACED	CHECKED
YBS.	R.R.L.	LGW.	R.J.L.
			8-30-66



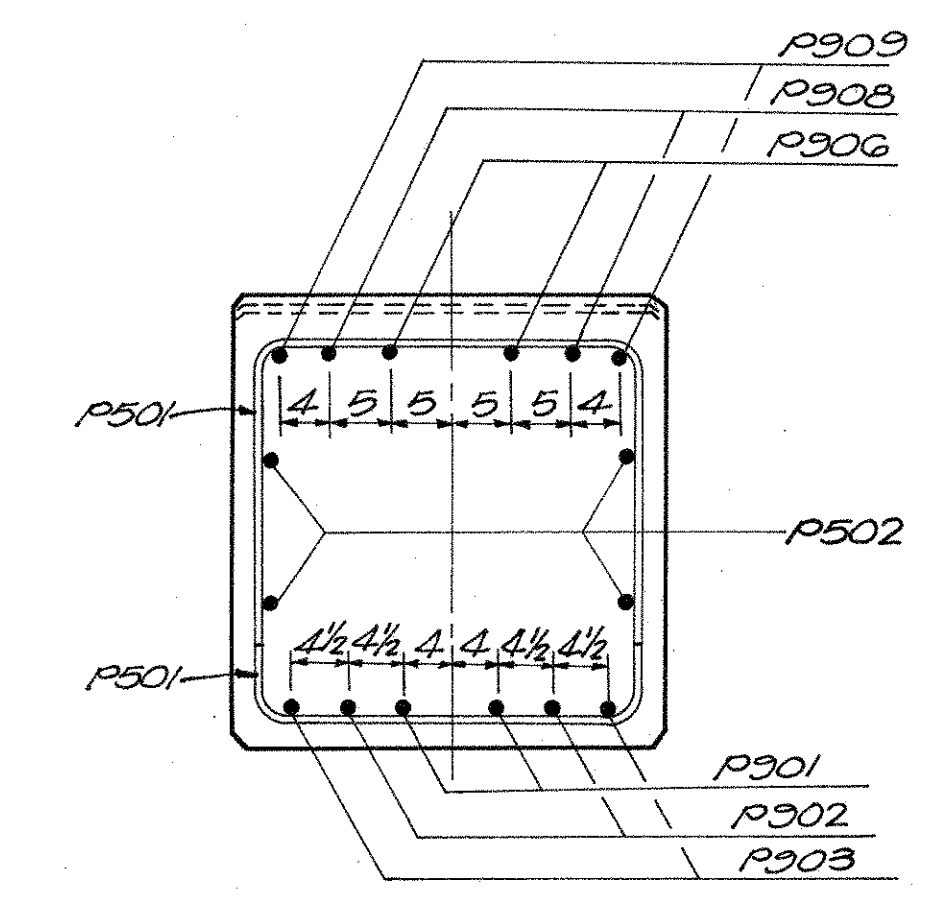
CAP PLAN



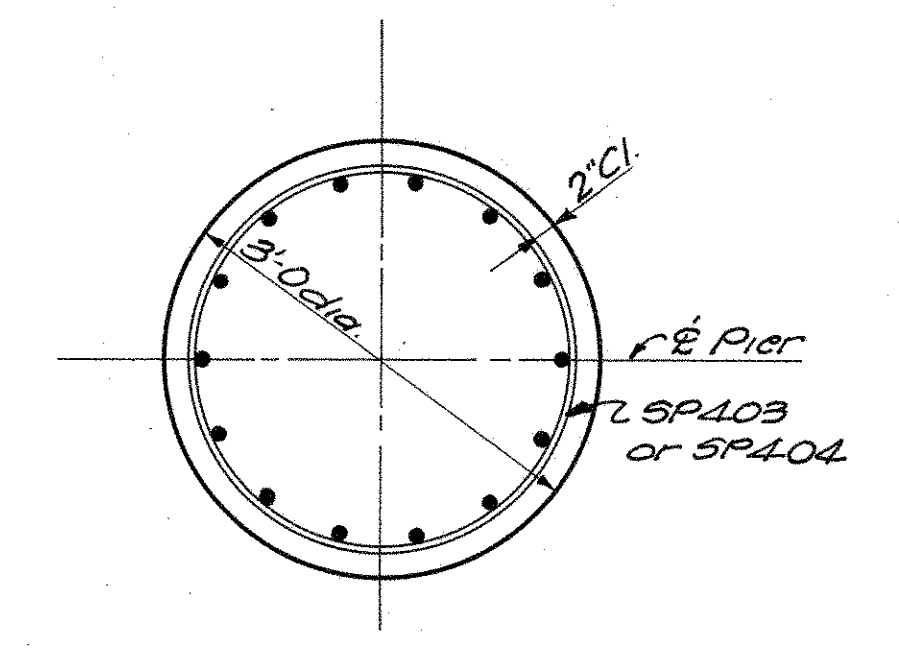
ELEVATION



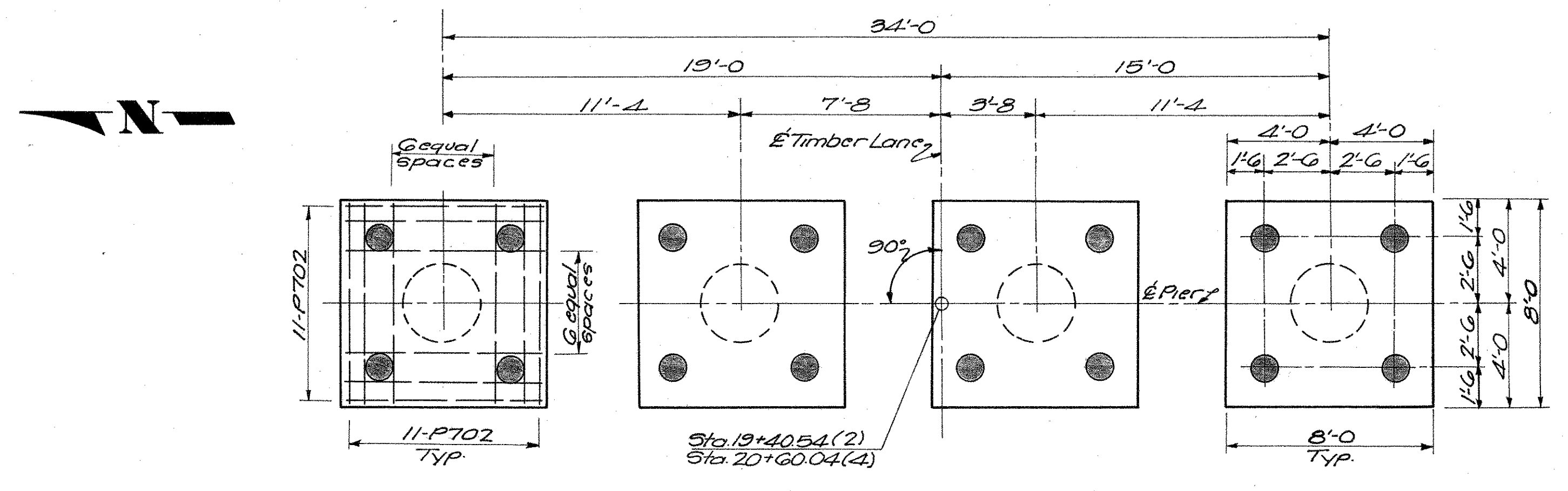
END VIEW



SECTION A-A

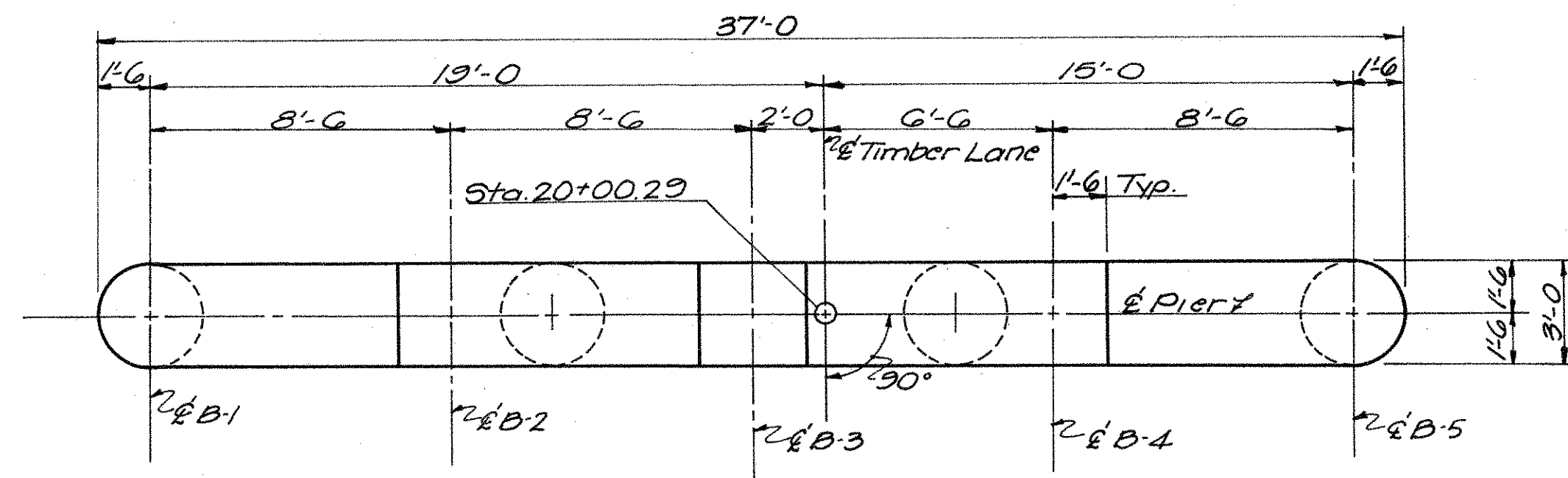


SECTION B-B

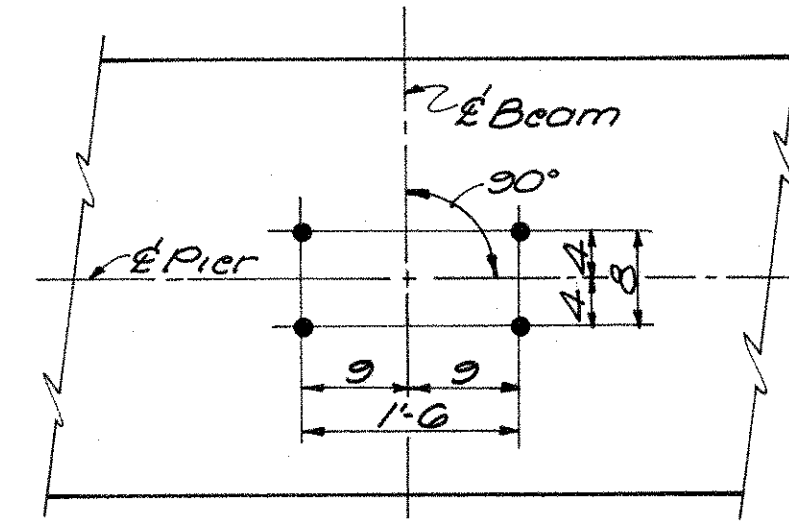


FOOTING PLAN

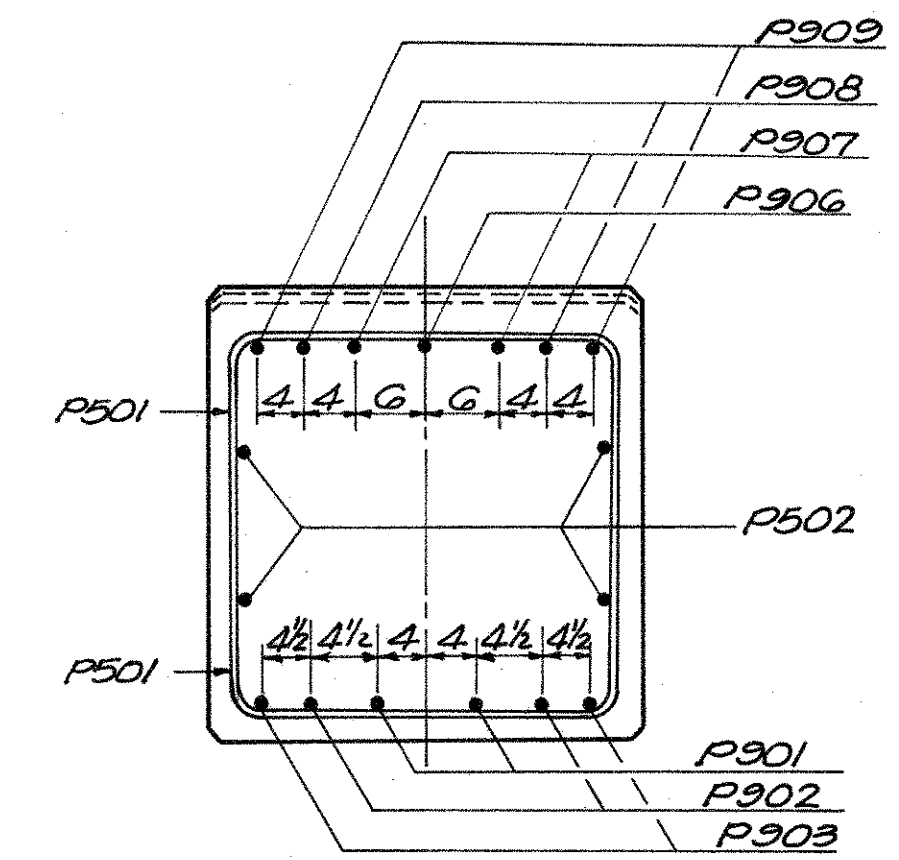
SHAW, LENZ & ASSOCIATES ENGINEERS					6/10
CINCINNATI, OHIO					
PIERS NO. 2 & NO. 4					
BRIDGE NO. MOT-75-1831 TIMBER LANE OVER I-75					
1-75					
MONTGOMERY CO. STA. 107+25.78					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	R.R.L.	L.G.W.	R.J.L.		8-30-66



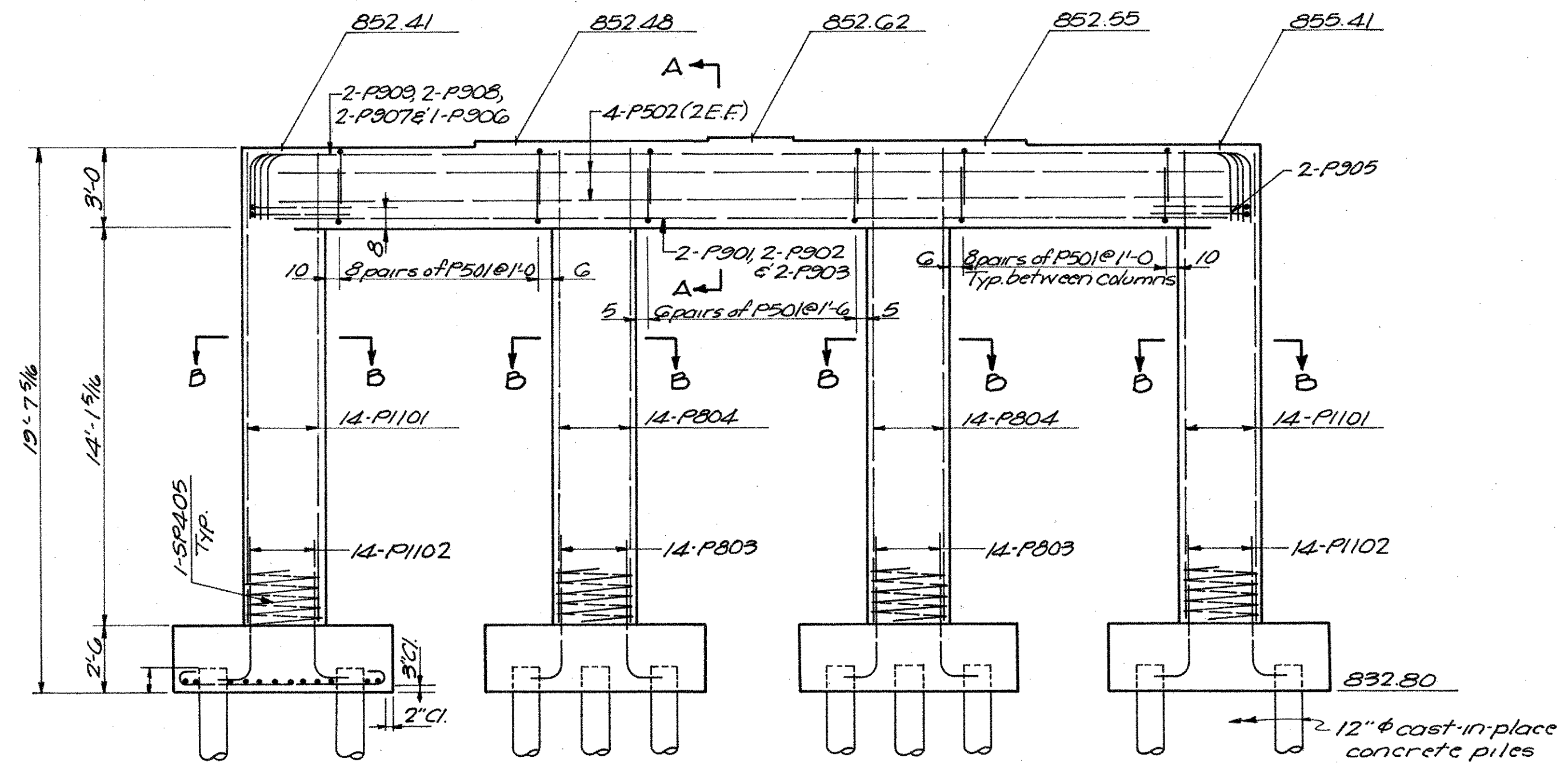
CAP PLAN



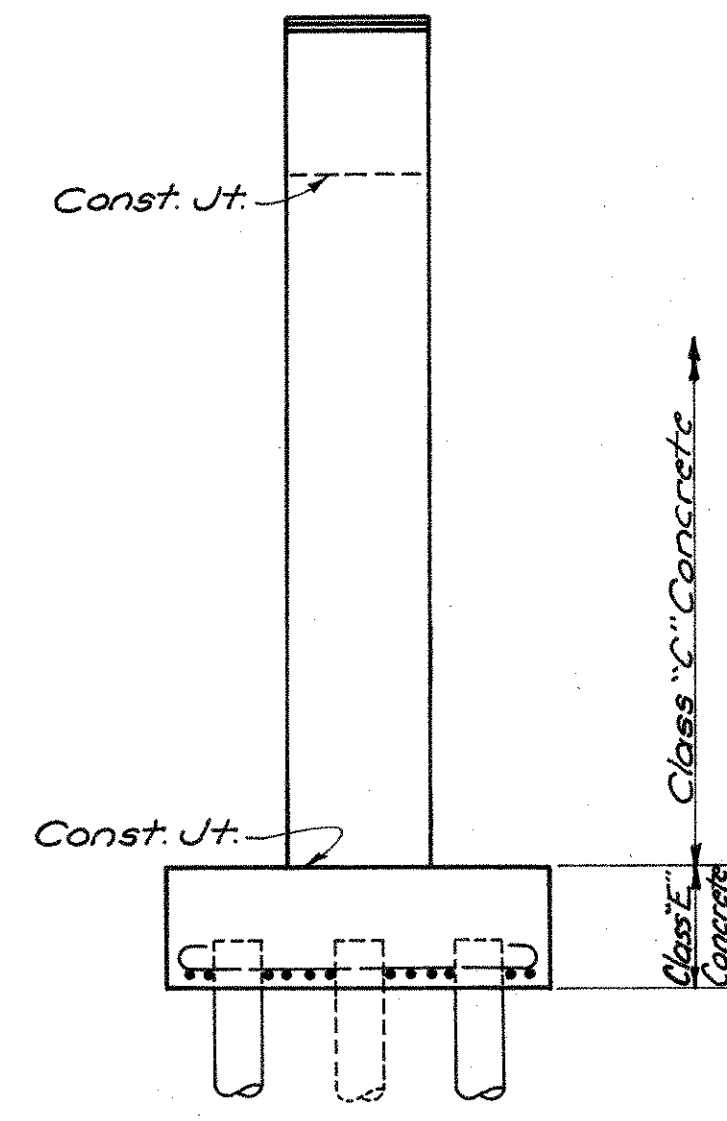
ANCHOR BOLT LAYOUT



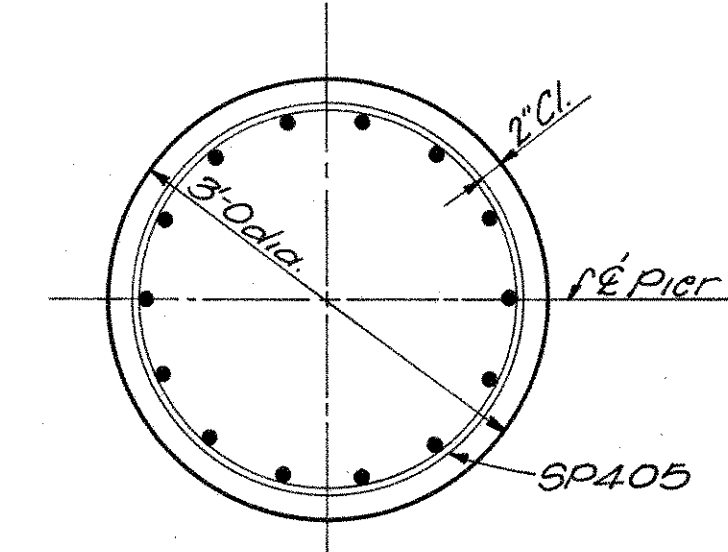
SECTION A-A



ELEVATION

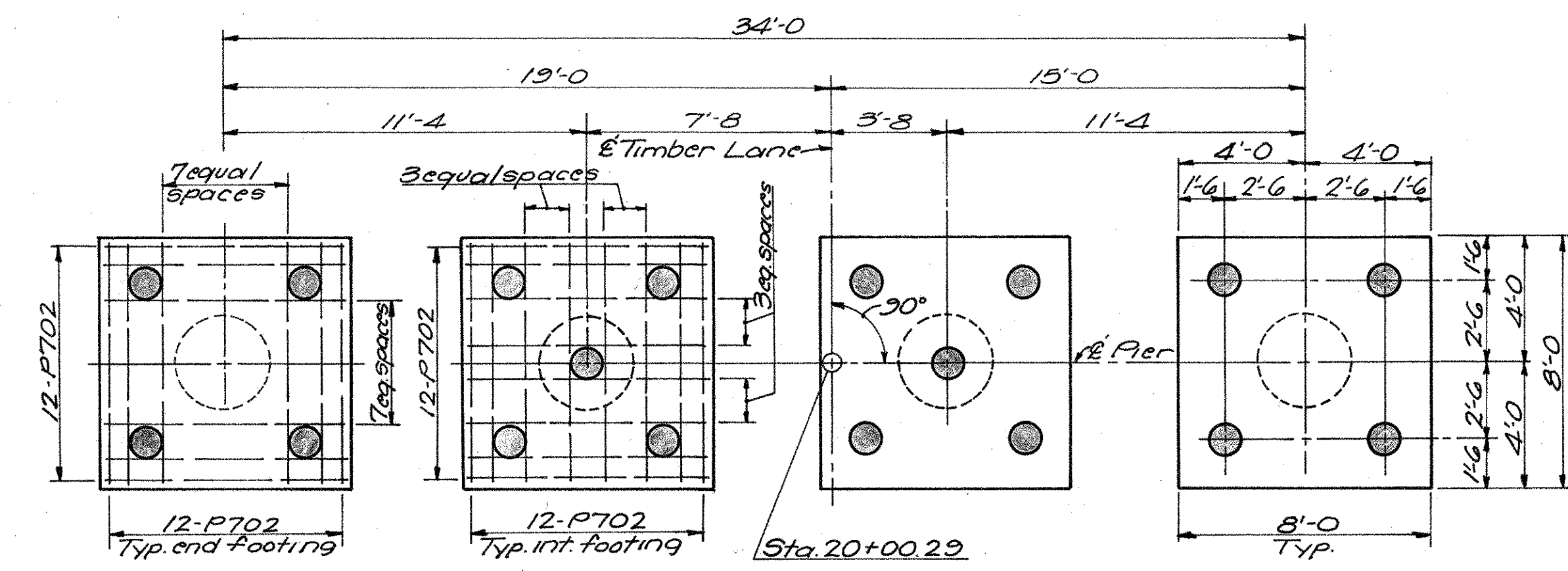


END VIEW



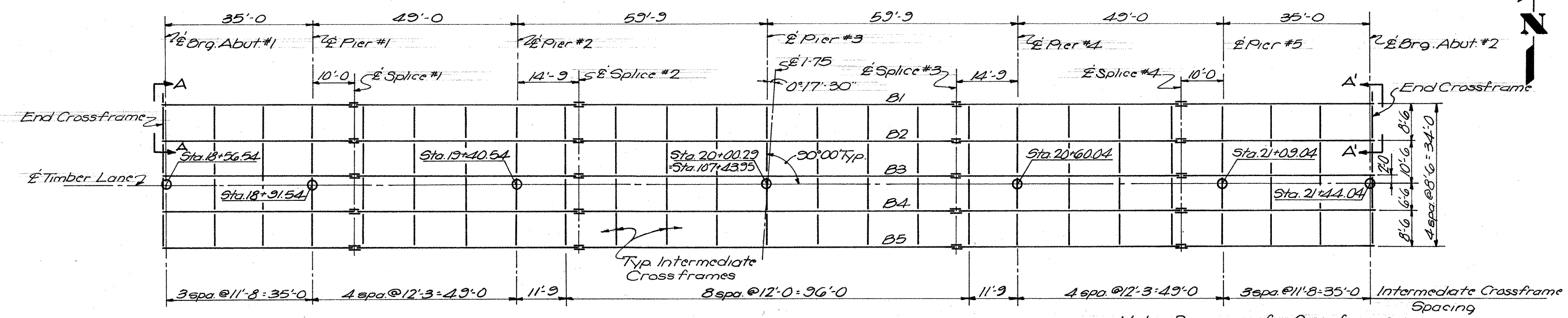
SECTION B-B

NOTES:
 Special care shall be taken in placing reinforcing steel in top of pier cap so as to avoid interference with the drilling of anchor rod holes.



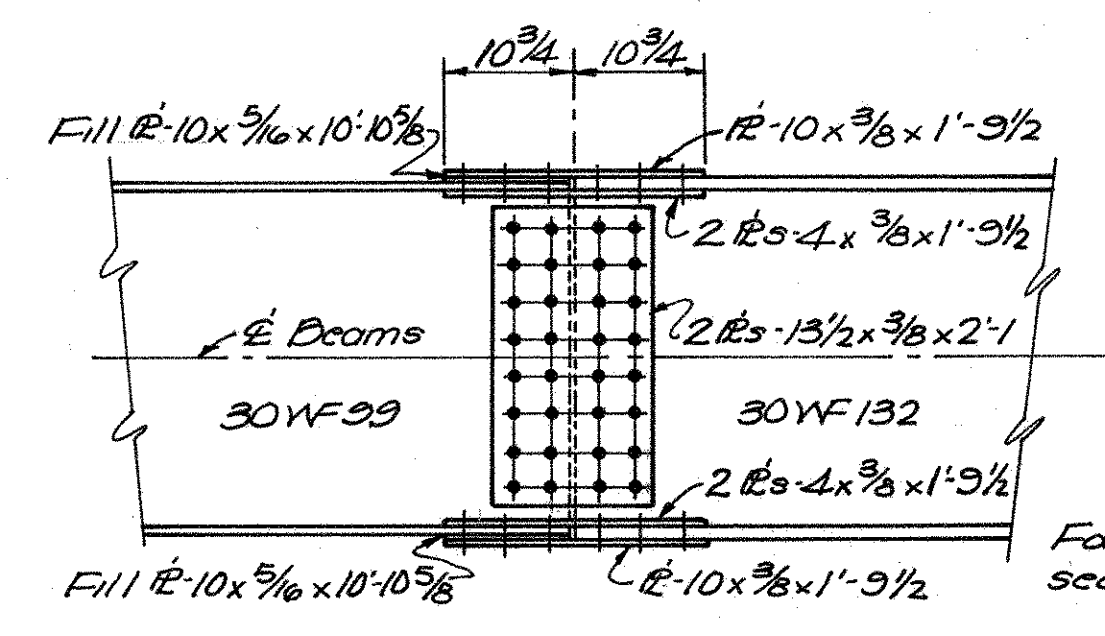
FOOTING PLAN

SHAW, LENZ & ASSOCIATES 7/10				
ENGINEERS				
CINCINNATI				OHIO
PIER NO. 3				
BRIDGE NO. MOT-75-1831				
TIMBER LANE OVER I-75				
1-75				
MONTGOMERY CO.			STA. 107+25.78	
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
W.B.S.	R.R.L.	W.T.R.	R.J.L.	
			8-30-66	



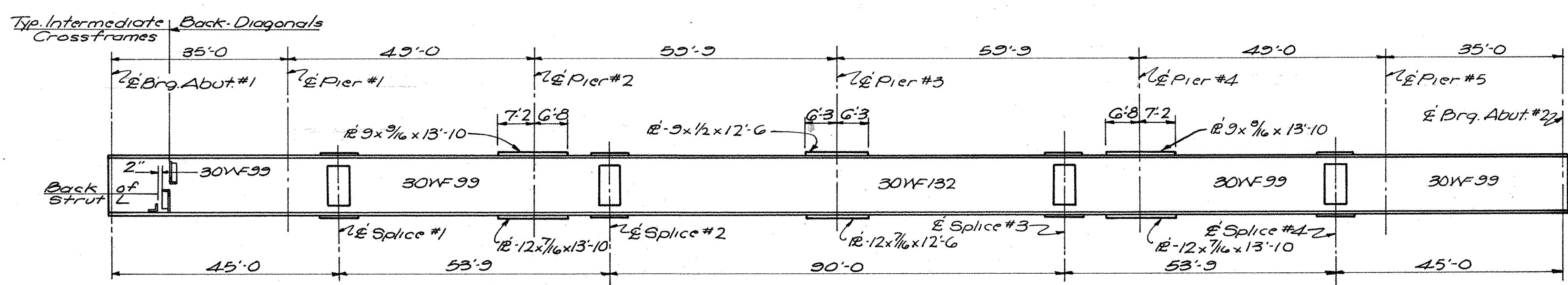
FRAMING PLAN

Note: Dimensions for Crossframes are to back of diagonals

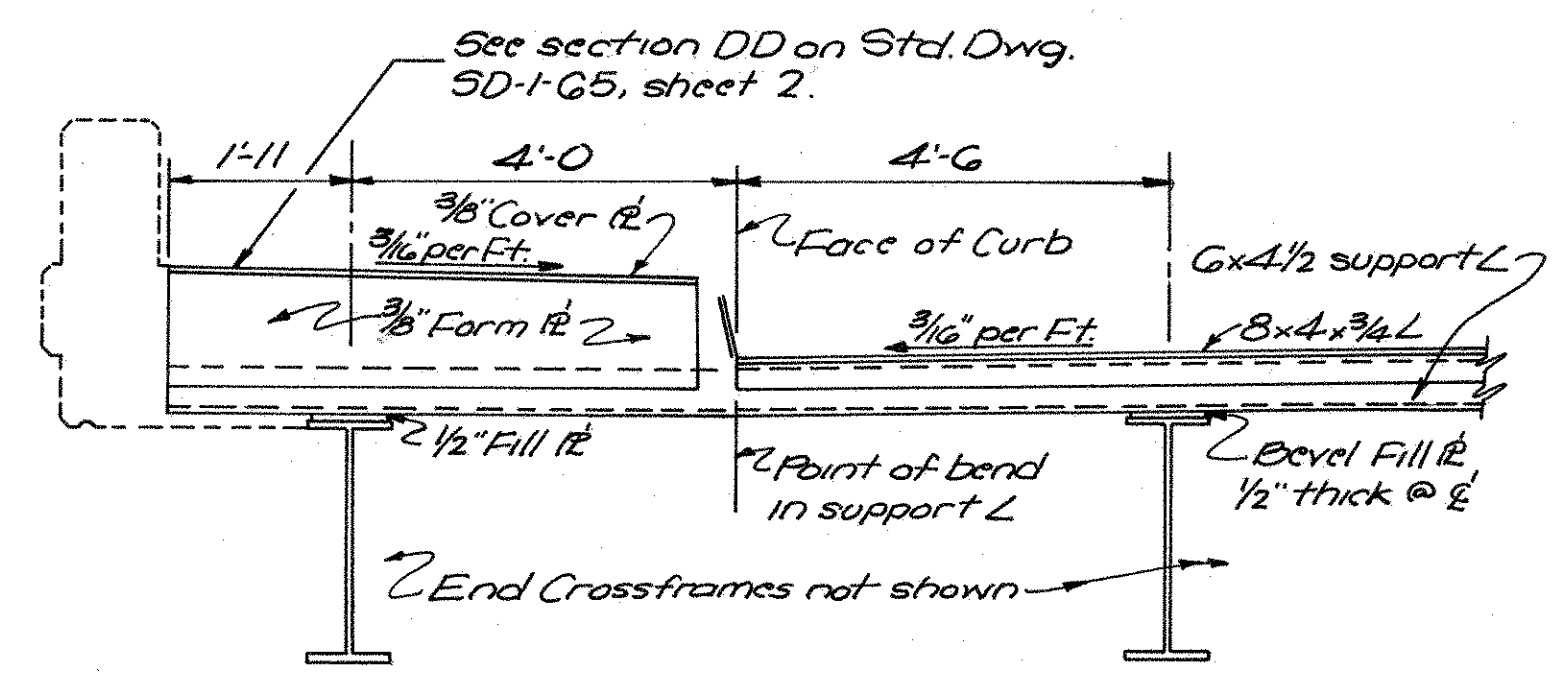


SPLICE DETAIL

For details and dimensions see Std. Dwg. 5D-1-G5 sh. 3
Splices #2 & #3 shown, splices #1 & #4 are same 1/2" fill rls.



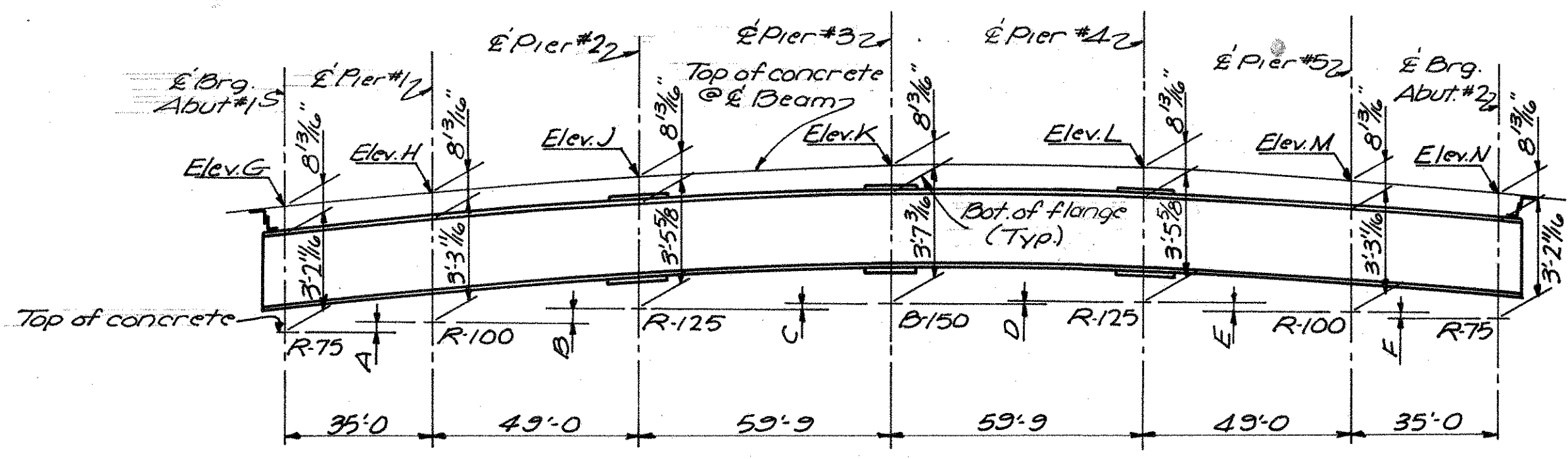
TYPICAL BEAM ELEVATION



SECTION A-A

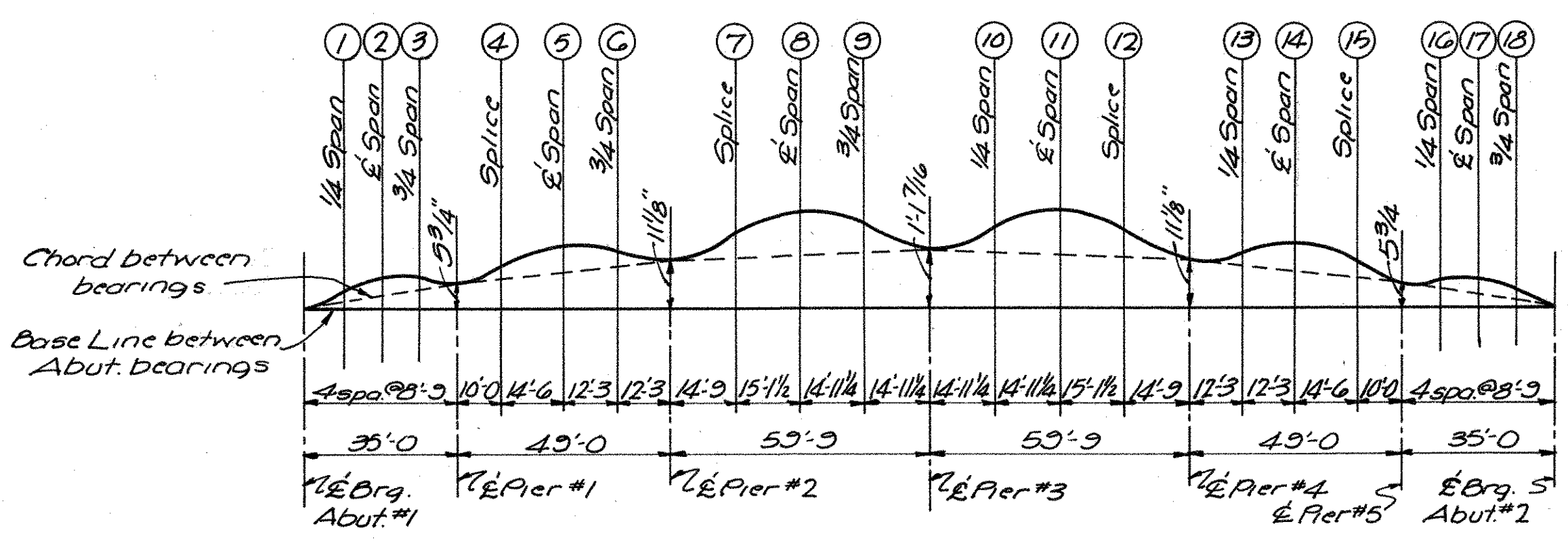
(Section A-A similar)

Note: Grade @ Abut. #1 = +1.77%
Grade @ Abut. #2 = -1.37%



ELEVATION LAYOUT

Beam	Dim. A	Dim. B	Dim. C	Dim. D	Dim. E	Dim. F	E.I. G	E.I. H	E.I. J	E.I. K	E.I. L	E.I. M	E.I. N
B-1	5 1/4	4 1/4	2 1/4	3/8	2 1/4	3 1/4	855.34	855.89	856.43	856.75	856.67	856.32	855.91
B-2	5 1/4	4 1/4	2 1/8	5/8	2 1/4	3 1/4	855.41	855.96	856.51	856.82	856.74	856.39	855.98
B-3	5 1/4	4 1/4	2 1/8	5/8	2 1/4	3 1/4	855.54	856.09	856.64	856.95	856.87	856.52	856.11
B-4	5 1/4	4 1/4	2 1/8	5/8	2 1/4	3 1/4	855.47	856.02	856.57	856.88	856.80	856.45	856.04
B-5	5 1/4	4 1/4	2 1/4	5/8	2 1/4	3 1/4	855.34	855.89	856.43	856.75	856.67	856.32	855.91

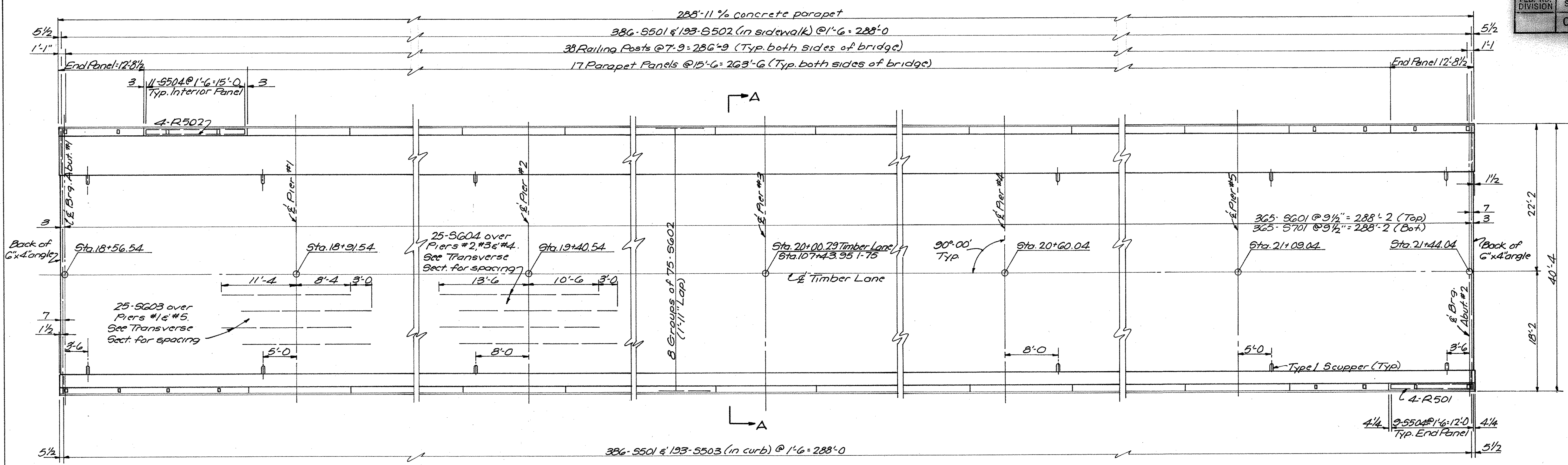


Beam	Location	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
B1	Deflection due to DL Beam	0	0	0	0	1/32	0	1/32	1/16	1/32	1/32	1/16	1/32	0	1/32	0	0	0	0
	Deflection due to remaining DL	3/32	3/32	1/32	3/32	3/16	3/32	7/32	3/8	3/16	3/16	7/32	3/32	3/16	3/32	1/32	3/32	3/32	3/32
	Vertical Curve Correction	5/32	3/16	5/32	1/4	1/32	7/32	7/16	1/32	7/16	7/16	1/32	7/16	3/32	1/32	1/4	5/32	3/16	5/32
	Required Shop Camber	1/4	5/16	3/16	3/8	5/8	3/8	1/16	1/16	1/16	1/16	1/16	1/16	3/8	5/8	3/8	3/16	5/16	1/4
B2	Deflection due to DL Beam	0	0	0	0	1/32	0	1/32	1/16	1/32	1/32	1/16	1/32	0	1/32	0	0	0	0
	Deflection due to remaining DL	3/32	3/32	1/32	3/32	3/16	3/32	7/32	3/8	3/16	3/16	7/32	3/32	3/16	3/32	1/32	3/32	3/32	3/32
	Vertical Curve Correction	5/32	3/16	5/32	1/4	1/32	7/32	7/16	1/32	7/16	7/16	1/32	7/16	3/32	1/32	1/4	5/32	3/16	5/32
	Required Shop Camber	1/4	5/16	3/16	3/8	5/8	3/8	1/16	1/16	1/16	1/16	1/16	1/16	3/8	5/8	3/8	3/16	5/16	1/4

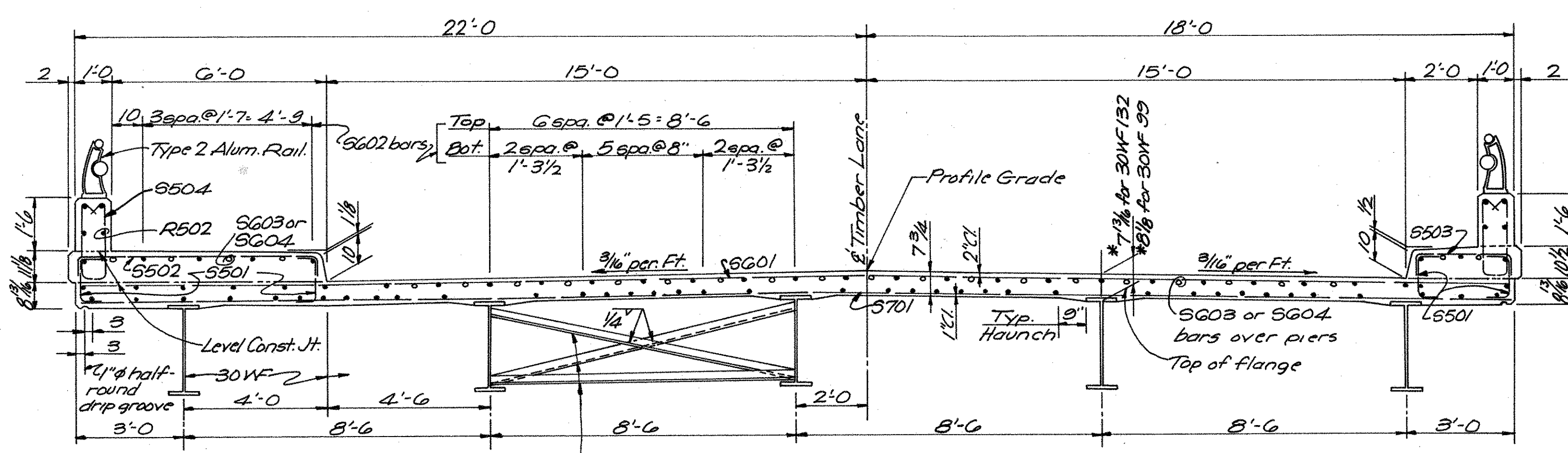
DEFLECTION & CAMBER DIAGRAM

NOTES:
 For Scupper details, see Std. Dwg. 5D-1-G5, sheet 2.
 For Scupper locations, see sheet 3.
 For end cross frames, roadway end dams, sidewalk end dam, and curb plate details, see Std. Dwg. 5D-1-G5, sheets 1 & 2.
 For details of Rockers and Bolsters, see Std. Dwg. RB-1-55.
 For intermediate crossframes see sheet 3.
 For shop welding of moment plates, see Std. Dwg. 5D-1-G5 sheet 2.

SHAW, LENZ & ASSOCIATES		8/10
ENGINEERS		OHIO
FRAMING PLAN		
BRIDGE NO. MOT. -75-1831 TIMBER LANE OVER I-75		
MONTGOMERY CO.		STA. 107+25.78
DESIGNED	DRAWN	TRACED
W.B.S.	R.R.L.	W.T.R.
		8-20-76
CHECKED	REVIEWED	REVISED
R.J.L.	W.B.S.	



PLAN

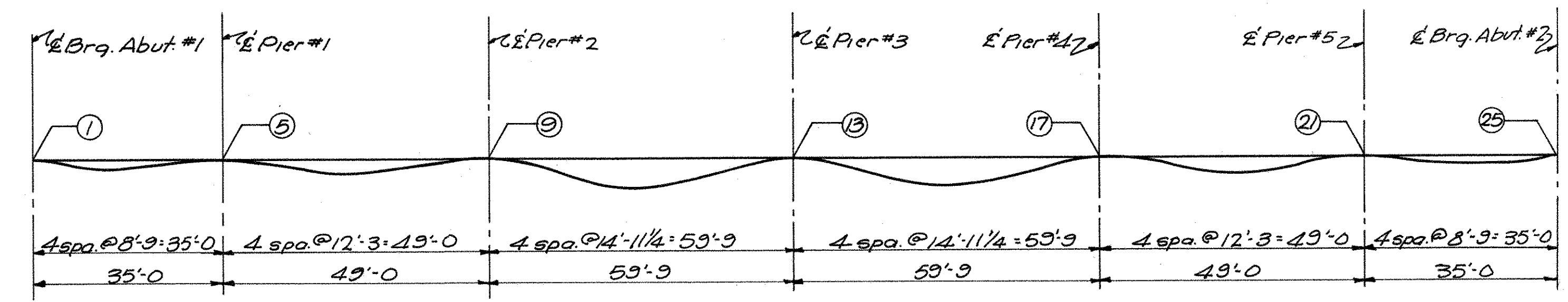


TRANSVERSE SECTION A-A

Intermediate crossframe angles 3x3x5/16. Weld both sides of vertical leg and top side of horizontal leg to beam with 1/4" continuous fillet weld.

DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of steel beams, which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12", except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width.

*This is a nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade.



SCREED ELEVATION DIAGRAM

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25		
A	18+ 56.54	18+ 65.29	18+ 74.04	18+ 82.79	18+ 91.54	19+ 03.79	19+ 16.04	19+ 28.29	19+ 40.54	19+ 52.79	19+ 65.04	19+ 77.29	19+ 89.54	20+ 01.79	20+ 14.04	20+ 26.29	20+ 38.54	20+ 50.79	20+ 63.04	20+ 75.29	20+ 87.54	21+ 00.79	21+ 13.04	21+ 25.29	21+ 37.54	21+ 49.79	
B	855.76	855.72	855.67	855.60	855.52	855.43	855.33	855.22	855.11	854.99	854.87	854.75	854.63	854.51	854.39	854.27	854.15	854.03	853.91	853.79	853.67	853.55	853.43	853.31	853.19	853.07	852.95
C	0.000	0.007	0.008	0.003	0.000	0.009	0.012	0.006	0.000	0.017	0.027	0.014	0.000	0.014	0.027	0.017	0.000	0.006	0.012	0.009	0.000	0.003	0.008	0.007	0.000	0.000	
D	855.34	855.50	855.64	855.77	855.89	856.06	856.21	856.33	856.44	856.56	856.67	856.72	856.75	856.78	856.74	856.74	856.67	856.61	856.54	856.44	856.32	856.23	856.14	856.03	855.91	855.79	855.67
E	0.000	0.007	0.008	0.003	0.000	0.009	0.014	0.006	0.000	0.017	0.028	0.015	0.000	0.015	0.028	0.017	0.000	0.006	0.012	0.009	0.000	0.003	0.008	0.007	0.000	0.000	
F	855.34	855.50	855.64	855.77	855.89	856.06	856.21	856.33	856.44	856.56	856.67	856.72	856.75	856.78	856.74	856.67	856.61	856.54	856.44	856.32	856.23	856.14	856.03	855.91	855.79	855.67	855.55
G	855.34	855.50	855.64	855.77	855.89	856.06	856.21	856.33	856.44	856.56	856.67	856.72	856.75	856.78	856.74	856.67	856.61	856.54	856.44	856.32	856.23	856.14	856.03	855.91	855.79	855.67	855.55

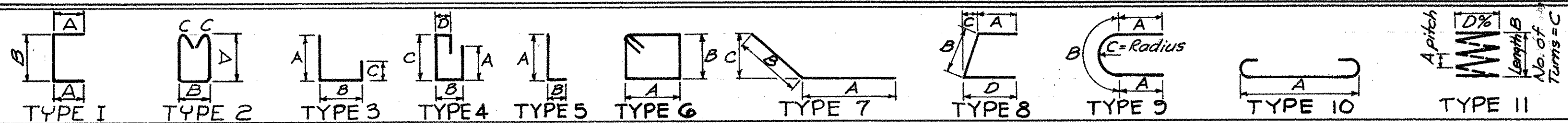
A = Station
 B = Profile Grade
 C = Gutter Elev. 15' Rt. & Lt.
 D = Concrete D.L. Defl. Rt. Gutter
 E = Screed Setting Elev. Rt. Gutter
 F = Concrete D.L. Defl. Lt. Gutter
 G = Screed Setting Elev. Lt. Gutter

NOTES

- Slab thickness includes 1" monolithic wearing surface.
- Spread longitudinal reinforcing steel in slab to clear scuppers.
- For Scupper details, see Std. Dwg. SD-1-G5, sheet 2.
- For Railing and Parapet joint details, see Std. Dwg. BR-1-G5, sheet 2.
- For End Dam and curb plate details, see Std. Dwg. SD-1-G5 sheets 1 & 2.

SHAW, LENZ & ASSOCIATES ENGINEERS
 CINCINNATI, OHIO
SUPERSTRUCTURE SLAB
 BRIDGE NO. MOT-75-1831
 TIMBER LANE OVER I-75
 STA. 107+25.78
 MONTGOMERY CO.
 DESIGNED: W.B.S. DRAWN: R.R.L. TRACED: W.T.R. CHECKED: R.J.L. 8-30-66 REVIEWED: W.B.S.

REINFORCING STEEL BAR SCHEDULE



MARK	NO.	SIZE	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
SUPERSTRUCTURE										
5701	365		39'-8"	Str.					29594	Slab
5601	365		39'-8"	Str.					21746	Slab
5602	600		37'-9"	Str.					34020	Slab, sidewalk & curb
5603	50		19'-8"	Str.					1477	" " "
5604	75		24'-0"	Str.					2704	" " "
5501	772		2'-1"	1	0'-6"	1'-4"			1677	Slab, sidewalk & curb
5502	193		7'-3"	1	0'-6"	6'-6"			1459	Side walk
5503	193		3'-3"	1	0'-6"	2'-6"			654	Curb
5504	410		5'-7"	2	2'-2"	0'-8"	0'-5"		2388	Curb, sidewalk & parapet
*P501	16		12'-4"	Str.						Parapet
*P502	136		15'-2"	Str.						Parapet
ABUTMENTS										
A801	14		41'-4"	Str.					1545	Ftgs.
A802	24		9'-8"	Str.					619	"
A601	48		14'-3"	3	6'-7"	5'-5"	2'-7"		1027	Ftgs.
A602	72		14'-4"	4	4'-8"	1'-5"	6'-0"	0'-11"	1550	Backwall
A603	4		14'-0"	1	6'-7"	1'-2"			84	Ftgs.
A604	16		18'-2"	1	8'-8"	1'-2"			437	"
A501	28		6'-6"	Str.					190	Abut. Corners
A502	48		7'-2"	1	1'-0"	5'-5"			359	Ftgs.
A503	48		7'-5"	5	6'-7"	1'-2"			371	Seats & Ftgs.
A504	48		5'-2"	1	1'-0"	3'-5"			259	Seats
A505	34		39'-4"	Str.					1395	Seats & Backwalls
A507	36		5'-7"	2	2'-2"	0'-8"			210	Curbs & Sidewalks
A508	24		12'-3"	6	3'-2"	2'-7"			307	Ftgs.
A509	24		6'-8"	Str.					167	Wing walls
A510	8		11'-7"	7	6'-10"	3'-4"	1'-6"		97	"
A511	32		11'-8"	Str.					389	"
A512	12		4'-7"	Str.					57	Wing walls #2 & #4
A513	4		5'-4"	Str.					22	"
A514	2		10'-3"	1	4'-8"	1'-2"			21	"
A515	2		8'-11"	1	4'-0"	1'-2"			19	"
A516	6		8'-1"	1	3'-7"	1'-2"			51	"
A517	14		3'-2"	8	1'-0"	1'-4"	0'-2"	1'-2"	46	Curbs, Wing walls #2 & #4
A518	6		6'-1"	1	2'-6"	1'-4"			38	Wing walls #2 & #4
A519	2		5'-9"	1	2'-4"				12	"
A520	2		5'-1"	1	2'-0"				11	"
A521	2		4'-1"	1	1'-6"				9	"
A522	2		3'-1"	1	1'-0"	1'-4"			6	"
A523	6		7'-7"	3	4'-7"	0'-6"	2'-9"		47	Wing walls #1 & #3
A524	6		3'-0"	Str.					19	"
A525	2		8'-4"	3	5'-4"	0'-6"	2'-9"		17	"
A526	2		3'-9"	Str.					8	"
A527	2		12'-0"	4	3'-0"	1'-2"	4'-8"	0'-6"	25	"
A528	2		10'-8"	4	2'-4"	1'-2"	4'-0"	0'-6"	22	"
A529	6		9'-10"	4	1'-11"	1'-2"	3'-7"	0'-6"	62	"
*R503	12									Parapets & end posts
*R504	8									end posts
*R505	8									Parapets & end posts
*R506	16		11'-4"							Parapets

MARK	NO.	SIZE	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
PIERS										
P1101	28		16'-10"	Str.					2504	Cols., Pier #3
P1102	28		7'-2"	5	5'-8"	1'-9"			1066	Ftgs., Pier #3
P1001	48		17'-1"	Str.					3528	Cols., Pier #1
P1002	48		17'-0"	Str.					3511	Cols., Pier #5
P1003	152		6'-6"	5	5'-4"	1'-5"			4251	Ftgs., Piers #1, 2, 4 & 5
P1004	28		18'-3"	Str.					2199	Cols., Pier #2
P1005	28		18'-6"	Str.					2229	Cols., Pier #4
P901	10		36'-7"	Str.					1244	Caps
P902	10		36'-3"	Str.					1233	"
P903	10		35'-6"	Str.					1207	"
P904	8		8'-10"	9	2'-10"	3'-2"	1'-0 3/4"		240	Cap, Piers #1 & 5
P905	12		9'-1"	9	3'-5"	1'-1 3/4"			371	Cap, Piers #2, 3 & 4
P906	9		41'-7"	1		36'-4"			1272	Cap, Piers #1, 2, 3, 4 & 5
P907	2		41'-3"			36'-0"			281	Cap, Pier #3
P908	6		40'-11"			35'-8"			835	Cap, Piers #2, 3 & 4
P909	6		39'-8"			34'-5"			809	"
P910	4		40'-1"			34'-10"			545	Cap, Piers #1 & 5
P911	4		41'-1"	1	2'-10"	35'-10"			559	"
P801	28		18'-3"	Str.					1364	Cols., Pier #2
P802	28		18'-6"	Str.					1383	Cols., Pier #4
P803	84		5'-5"	5	4'-7"	1'-0"			1215	Ftgs., Piers #2, 3 & 4
P804	28		16'-10"	Str.					1258	Cols., Pier #3
P701	176		8'-10"	10	7'-2"				3178	Ftgs., Piers #1 & 5
P702	272		9'-4"	10	7'-8"				5189	Ftgs., Piers #2, 3 & 4
P501	188		6'-9"	1	2'-2"	2'-8"			1324	Caps
P502	20		34'-0"	Str.					709	"
SP401	4		14'-4 3/4"	11	4 1/2"	14'-4 7/8"	41	2'-8"	1060	Cols., Pier #1
SP402	4		14'-3 3/8"			14'-3 3/8"	41		1060	Cols., Pier #5
SP403	4		15'-6 3/8"			15'-6 3/8"	44		1136	Cols., Pier #2
SP404	4		15'-3 1/4"			15'-3 1/4"	45		1164	Cols., Pier #4
SP405	4		14'-1 3/8"	11	4 1/2"	14'-1 3/8"	41	2'-8"	1060	Cols., Pier #3

NOTES:

- All dimensions are out to out of bar
 - Radius dimensions are to outside of bar.
 - The length of bent bars is measured along centerline.
 - Bar size is indicated in the bar mark. The first digit where three digits are used and the first two digits where four are used indicates the bar size.
 - Four steel channel, tee or angle spacers weighing approximately 0.68 lbs. per lin. ft. of spacer shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantity for spiral bars. The length shown in the steel list for the spiral bars is the distance from the top of the footings to the bottom of the pier cap.
The "No. of Turns" shown is the "Length" divided by the pitch, plus 3 turns (total number of closed coils), expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item 509. 1/2 closed coils shall be provided at the ends of each spiral unit.
- * Included with Item 517 for payment.

MARK	NO.	LENGTH	TYPE
RE1101	1	8'-7"	Str.
RE1001	1	8'-3"	"
RE901	1	7'-10"	"
RE801	1	7'-6"	"
RE701	2	7'-3"	"
RE601	4	6'-11"	"
RE501	1	6'-7"	"
RE401	1	6'-3"	"

SHAW, LENZ & ASSOCIATES 10/10
ENGINEERS

CINCINNATI OHIO

REINFORCING STEEL DETAILS

BRIDGE NO. MOT. - 75 - 18 31
TIMBER LANE OVER I-75
MONTGOMERY CO. STA. 107 + 25.78

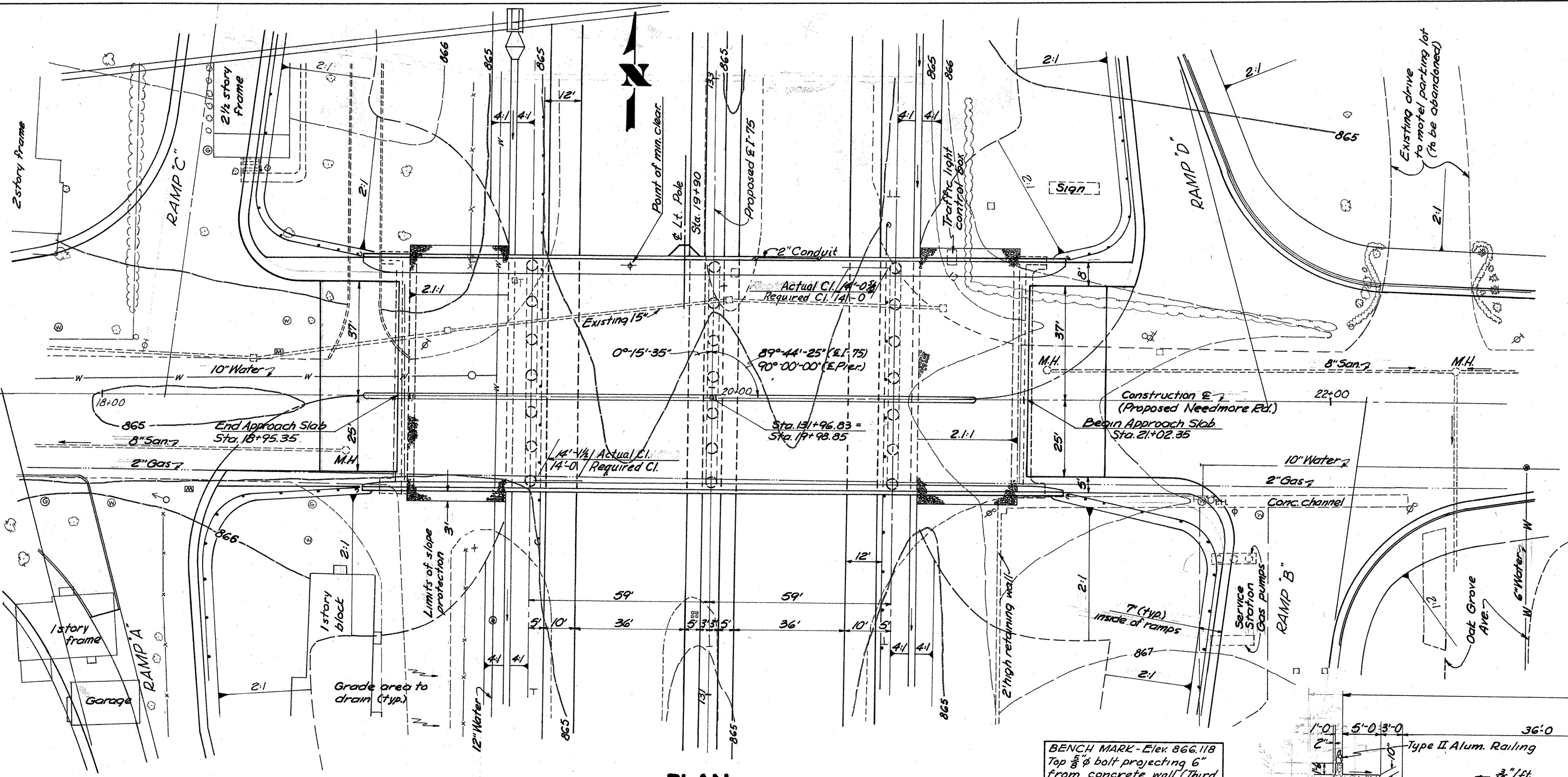
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
R.R.L.	R.R.L.	-	R.J.L.		

8-30-66

PROPOSED STRUCTURE
 Type: Continuous steel beam with reinf. concrete deck & substructure.
 Spans: 41'-6", 59'-9", 59'-9", & 41'-6".
 Roadway: 63'-0" w/ curbs, 2 lanes East-bound, 3 lanes West-bound with 2'-0" traffic divider & 1'-2'-0" safety curb & 1'-5'-0" sidewalk.
 Load Frequency: CF=2000 (57)
 Skew: 0°-00'-00"
 Wearing Surface: 1" monolithic concrete
 Approach Slab: AS-1-67 (25' long)
 Alignment: Tangent

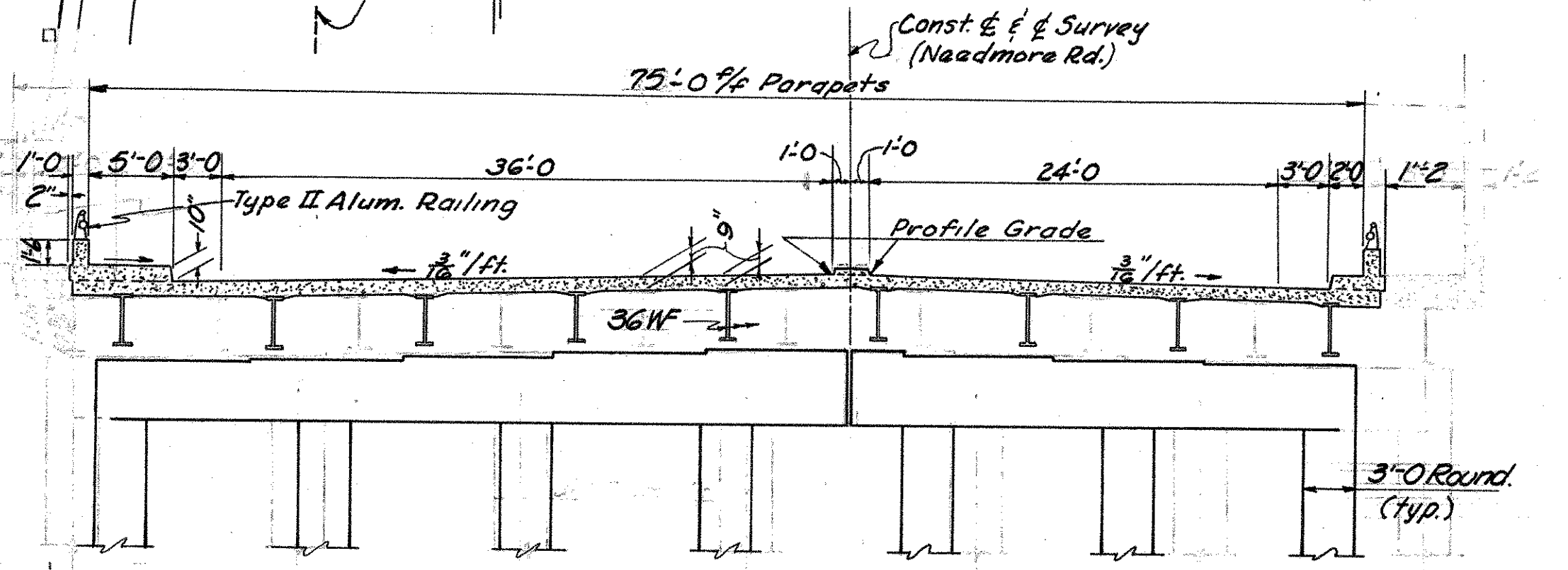
AVERAGE DAILY TRAFFIC - 1975
 A.D.T. 42,708 *
 Type "B" (Estimated) 2,135
 *Based on System 10-5 traffic survey prepared by Vogt-Ivers & Assoc., Feb. 1964.

Estimated pedestrian traffic 1975
 60 persons per day.

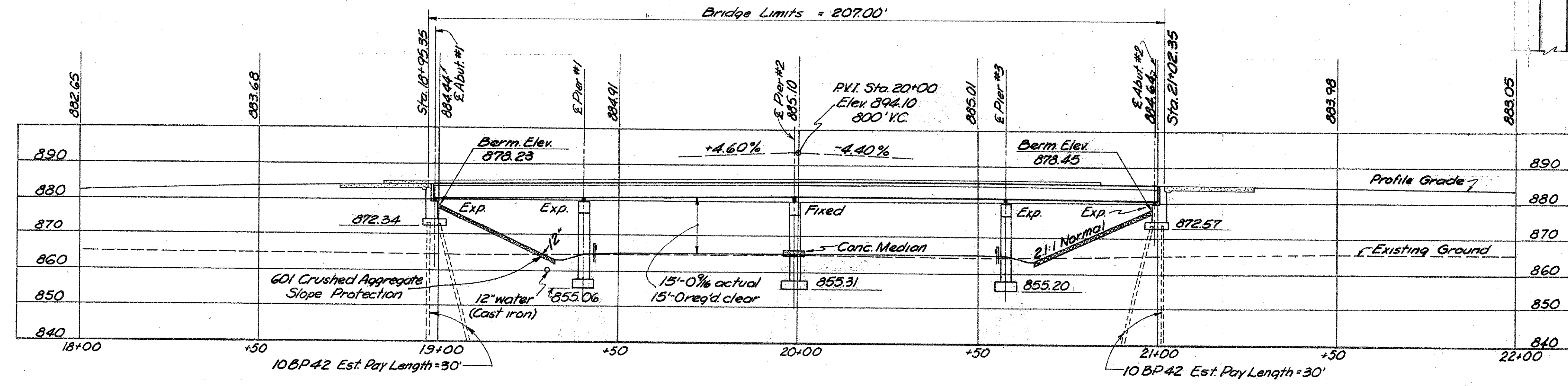


PLAN
 SCALE: 1"=20'

BENCH MARK - Elev. 866.118
 Top 3/8" bolt projecting 6" from concrete wall (Third bolt from N.) 32' Rt.
 Sta. 131+38.5



SECTION - BRIDGE DECK
 SCALE: 1/8"=1'-0"



SECTION ALONG E NEEDMORE RD.
 SCALE: 1"=20'

SHAW, LENZ & ASSOCIATES ENGINEERS
 CINCINNATI OHIO

SITE PLAN
 BRIDGE NO. MOT-75-1887
 NEEDMORE RD. OVER I-75
 MONTGOMERY CO.
 SCALE: As Shown

PRESENT TOPO SURVEYOR	PROPOSED WORK DESIGNED	WORK CHECKED	WORK REVIEWED
B.H.S.	ERB	W.B.S.	T.J.P. R.L.L.

All piers and abutments are parallel and 90° to centerline of Needmore Road.

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	ABUTS.	PIERS	SUPER STRUCTURE	GENERAL
503	1,042	Cu.Yds.	Unclassified Excavation	305	737		
503	Lump	Lump Sum	Coffer dams, Cribs and Sheeting		Lump		
505	Lump	Lump Sum	First Test Pile	Lump			
507	1,140	Lin.Ft.	Steel Piles (10BP42)	1,140			
509	21,481	Lbs.	Reinforcing Steel	16,445	6,184	133,622	
511	510	Cu.Yds.	Class "C" Concrete - Superstructure			510	
511	134	Cu.Yds.	Class "C" Concrete - Piers above footing		134		
511	150	Cu.Yds.	Class "C" Concrete - Abutments above footings	150			
511	267	Cu.Yds.	Class "C" Concrete - Footings	122	145		
512	14	Lin.Ft.	Premolded Sealing Strip	14			
513	330,575	Lbs.	Structural Steel			330,575	
514	330,575	Lbs.	Field Painting of structural steel			330,575	
517	455	Lin.Ft.	Bridge Railing Type 2	51		408	
518	51	Cu.Yds.	Porous Backfill				
518	139	Lin.Ft.	6" Perforated helical C.M.P. including specials 707.01	139			
518	100	Lin.Ft.	6" Non-perforated helical C.M.P. 707.01	100			
518	16	Each	Scuppers (including supports)			16	
601	662	Sq.Yds.	Crushed Aggregate Slope Protection				662
625			See lighting plans.				
808	510	Units	Water-reducing set-retarding admixture			510	

GENERAL NOTES

REFERENCE shall be made to the following:

Standard Drawings: BR-1-65 sheet 2 revised 11-24-65
 RB-1-55 revised 2-2-59
 SD-1-65 sheets 1, 2 & 3 dated 11-8-65
 AS-1-67 Dated 1-11-68
 Supplemental Specifications: 808 dated 1-1-69
 811 dated 1-1-69
 836 dated 6-11-69

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57 together with current revisions thereof.

DESIGN LOADING: CF-2000(57)

CONCRETE: Class "C" basic unit stress = 1333 p.s.i. for superstructure
 Class "C" basic unit stress = 1133 p.s.i. for substructure

STRUCTURAL STEEL: ASTM A-36 - basic unit stress = 20,000 p.s.i.

SCUPPERS including support angles, shall be galvanized in accordance with 711.02

REINFORCING STEEL: ASTM, A615, A616, A617, Deformed Intermediate or Hard. Basic unit stress 20,000 p.s.i. Spiral reinforcement shall be plain, A306 or A499

PILES shall be driven to a minimum bearing capacity of 35 tons per pile for the abutments.

FOUNDATION BEARING PRESSURE: Pier footings are designed for a maximum bearing pressure of 2.5 tons per sq. ft.

MACHINE FINISH: The concrete deck shall be finished by the use of a finishing machine.

UTILITY LINES: All expenses involved in relocating the affected utility lines shall be borne by the owners. The Contractors and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either is held to a minimum.

EXCAVATION QUANTITY includes the removal of fill materials required for construction of the abutments.

JOINT SEALER: Item 828 joint sealer including bond breaker, show in Section A-A of Std. Dwg. SD-1-65, Sheet No. 1 shall be omitted.

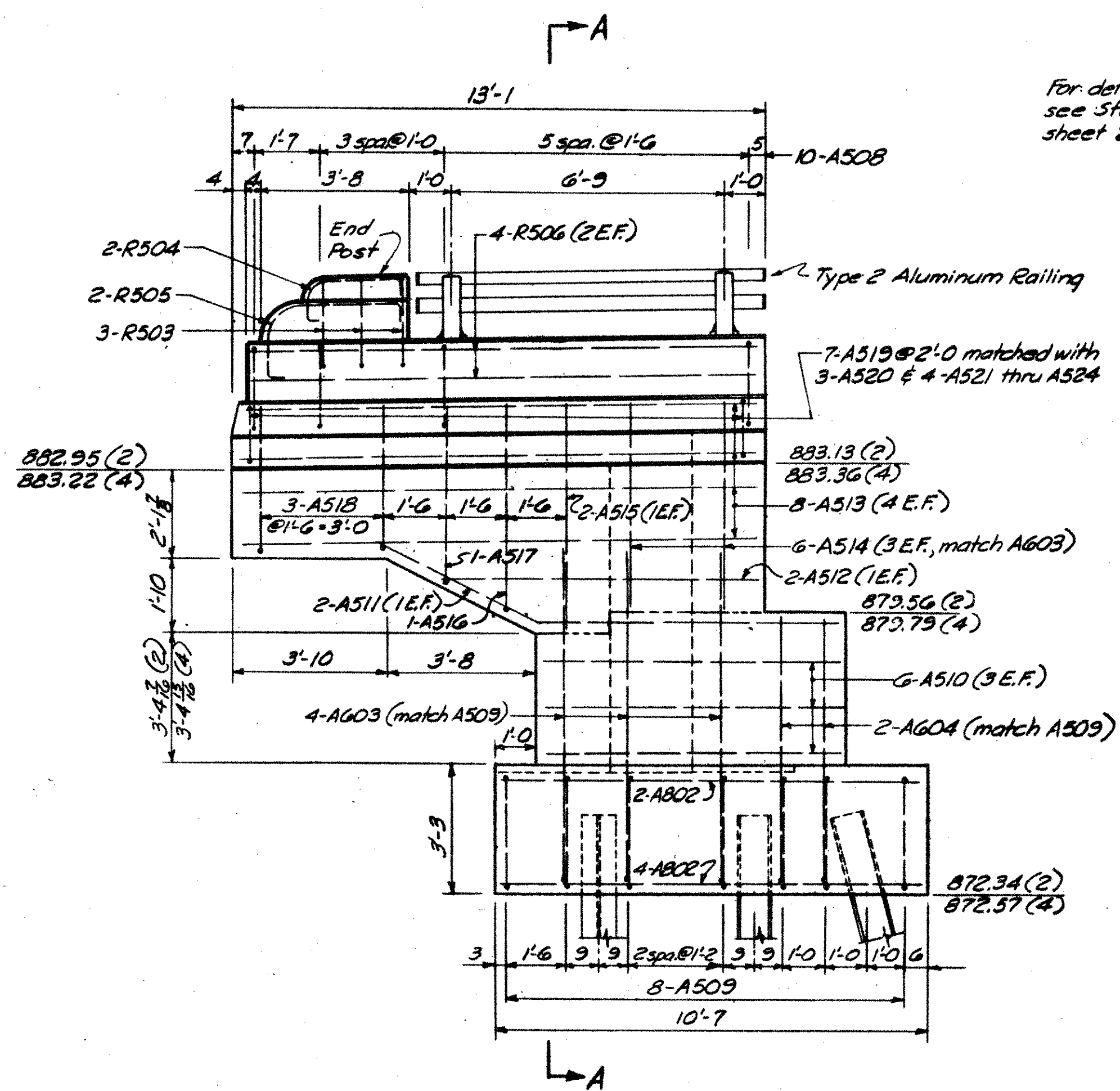
WELDS: Non-stress carrying welds are indicated thus:

SHAW, LENZ & ASSOCIATES ENGINEERS
 CINCINNATI OHIO

QUANTITIES & GENERAL NOTES

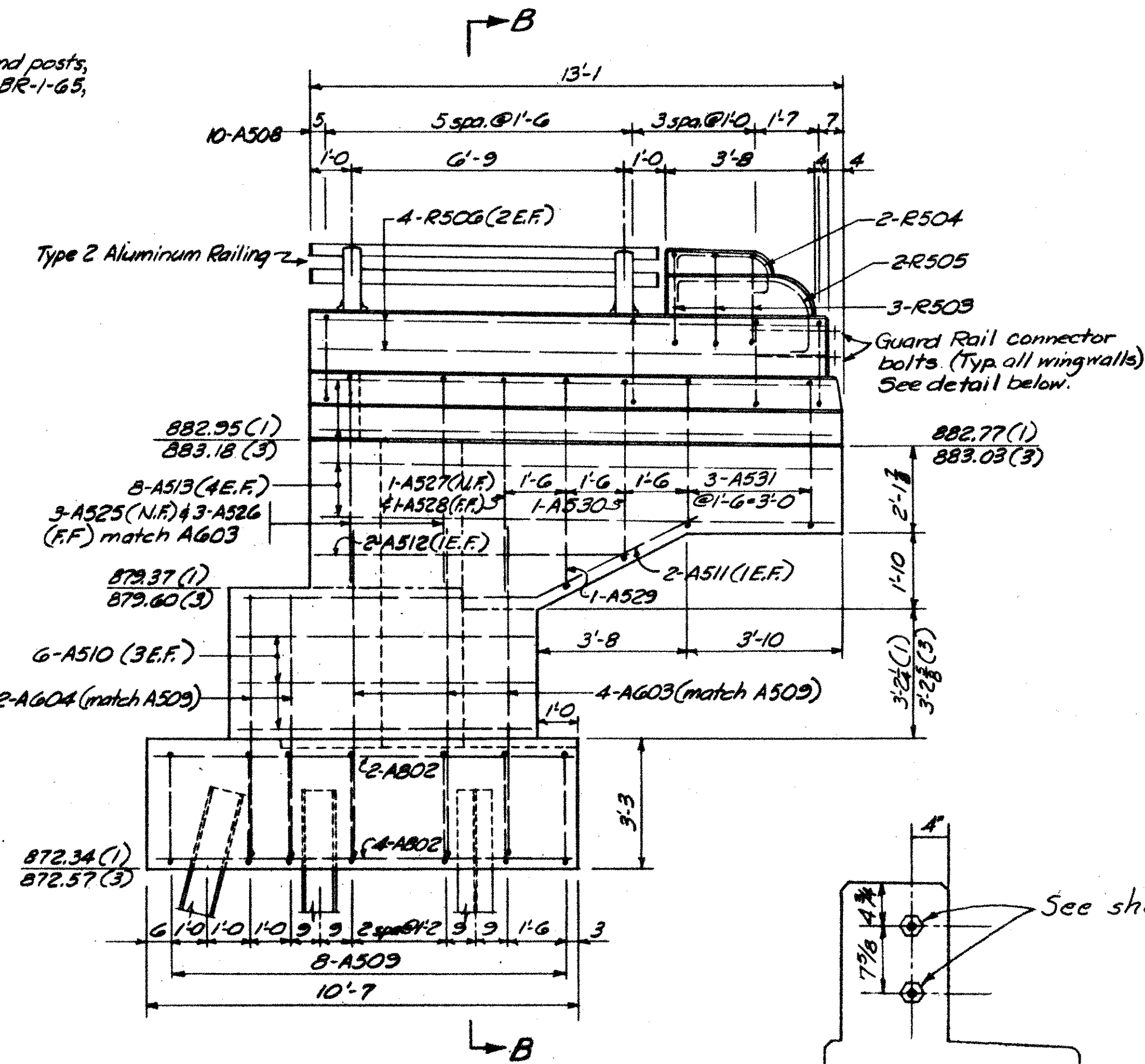
BRIDGE NO. MOT-75-1887
 NEEDMORE RD. OVER I-75

MONTGOMERY CO. STA. 135+65.66
 DESIGNED DRAWN TRACED CHECKED REVIEWED REVISIONS
 W.B.S. W.T.R. W.B.S. 8-5-66



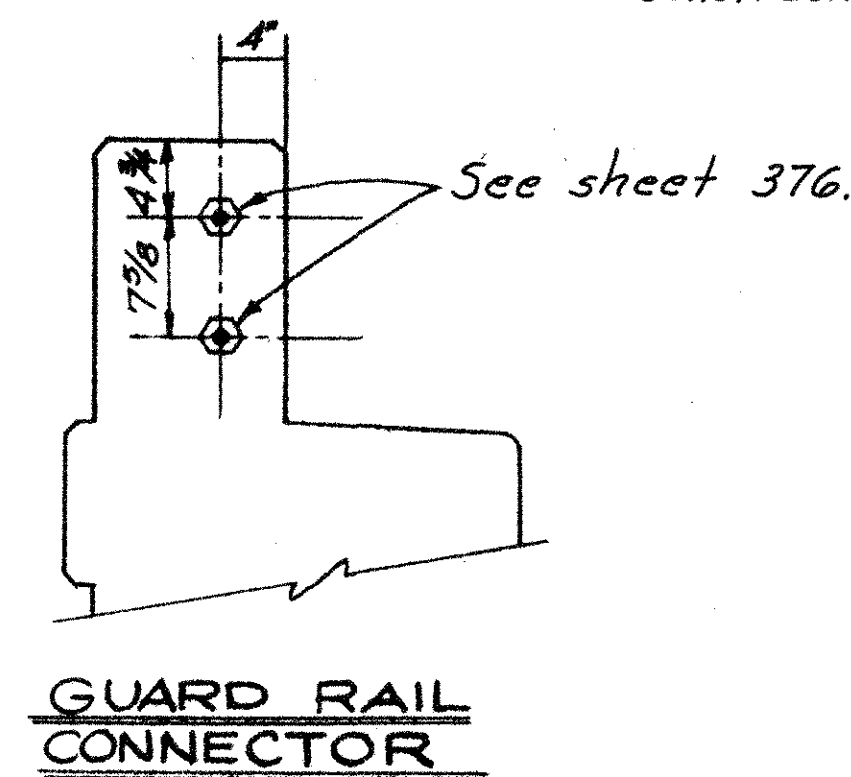
WINGWALLS NO.2 & NO.4

Wingwall #2 shown, Wingwall #4 is opposite hand

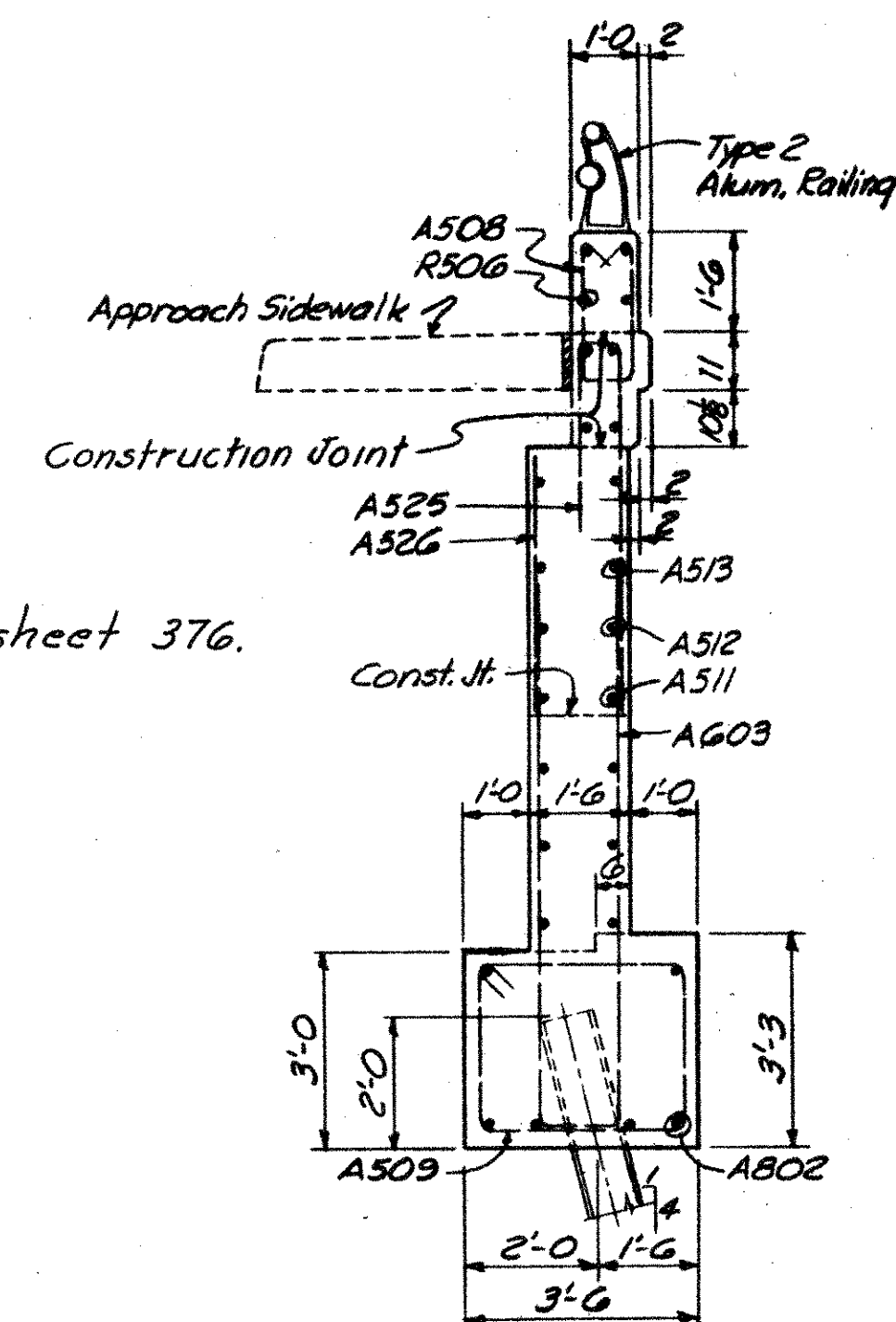


WINGWALLS NO.1 & NO.3

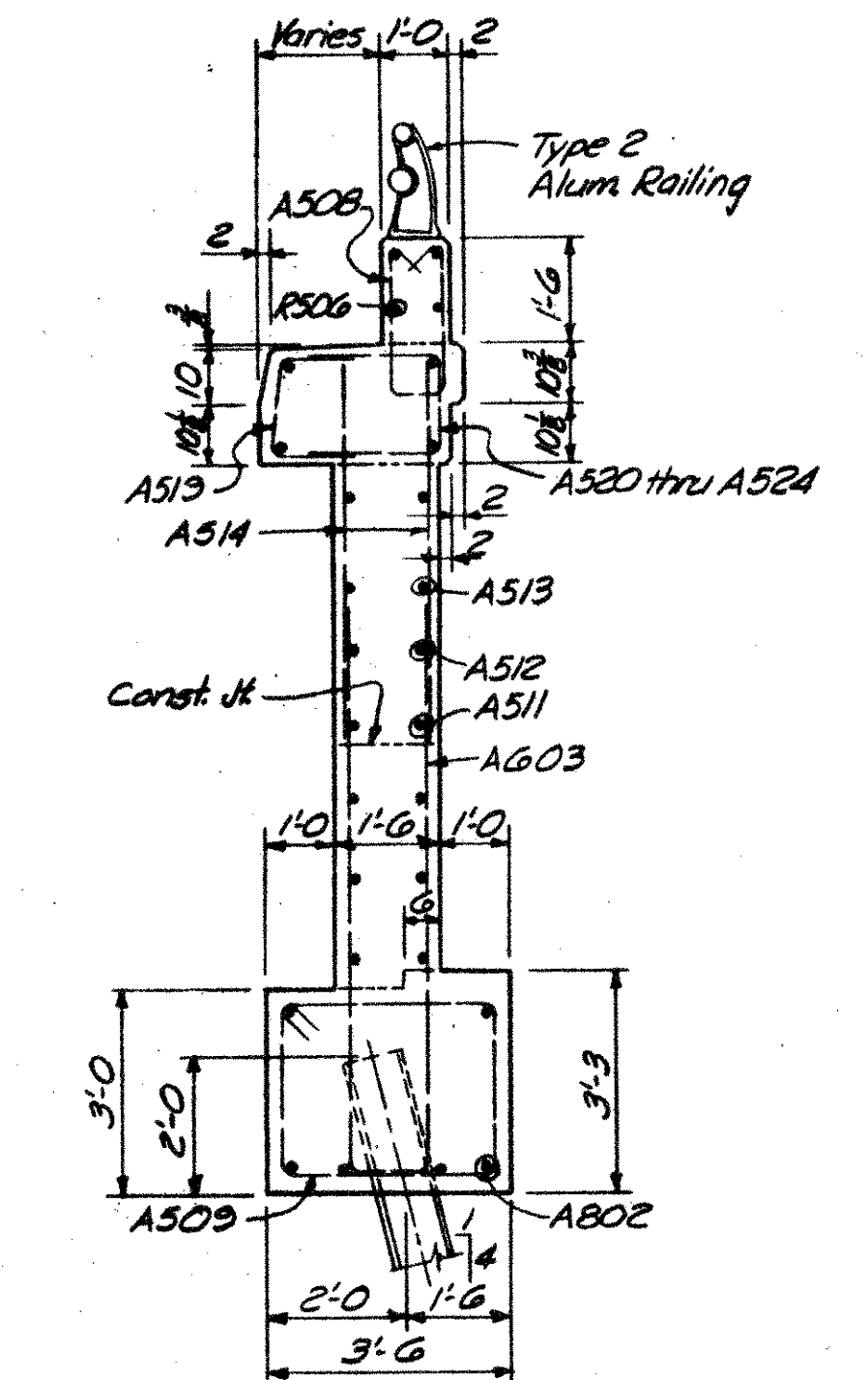
Wingwall #1 shown, Wingwall #3 is opposite hand



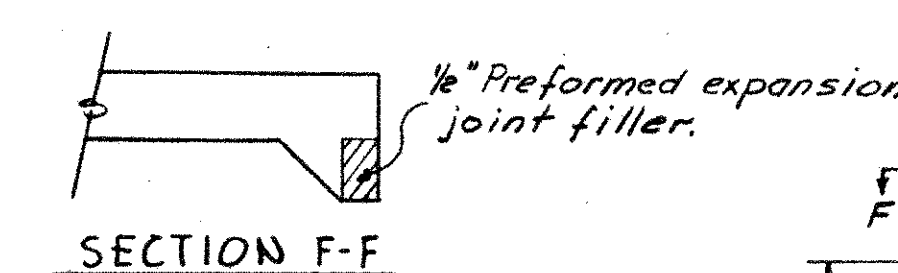
GUARD RAIL CONNECTOR



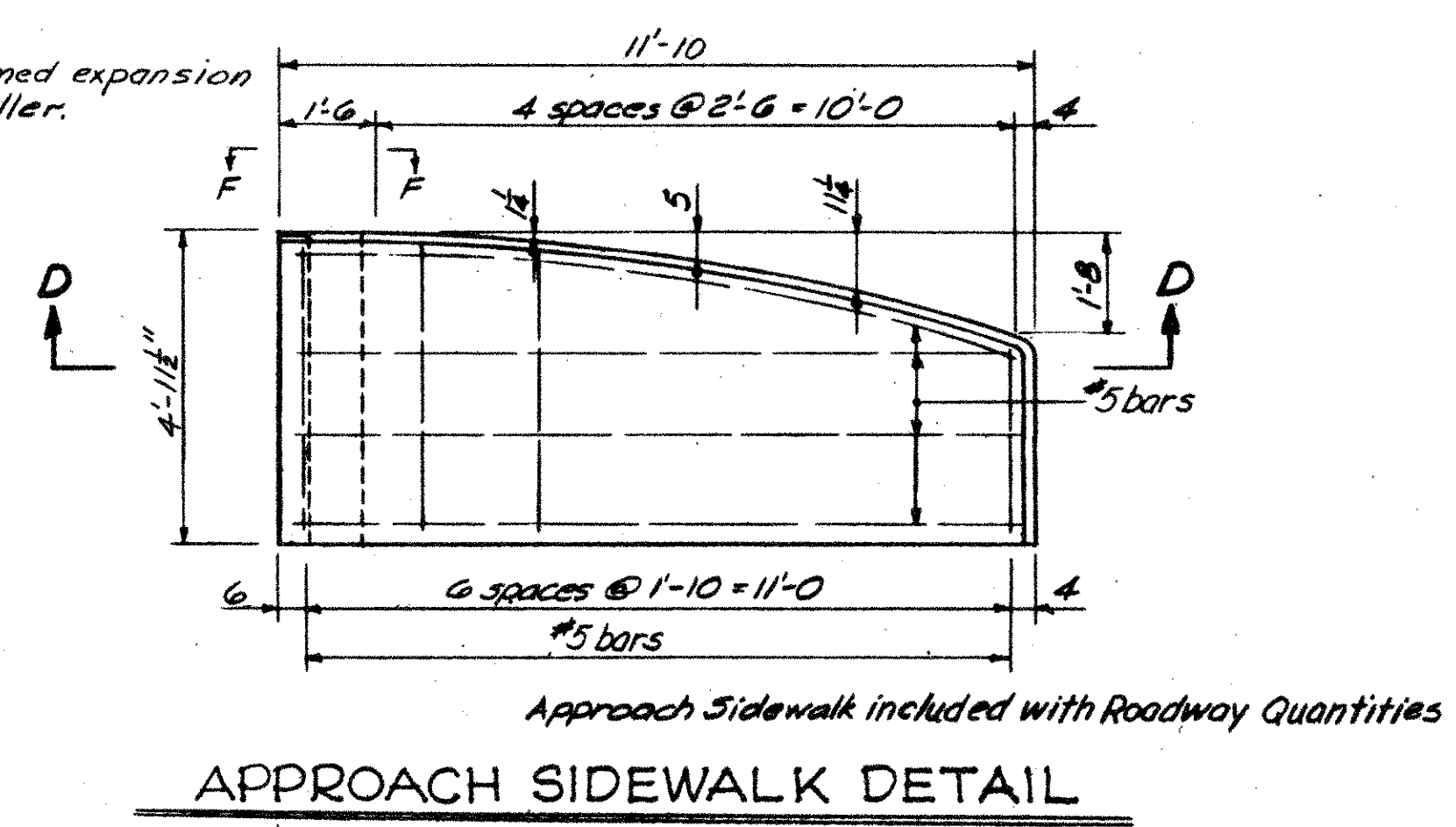
SECTION B-B



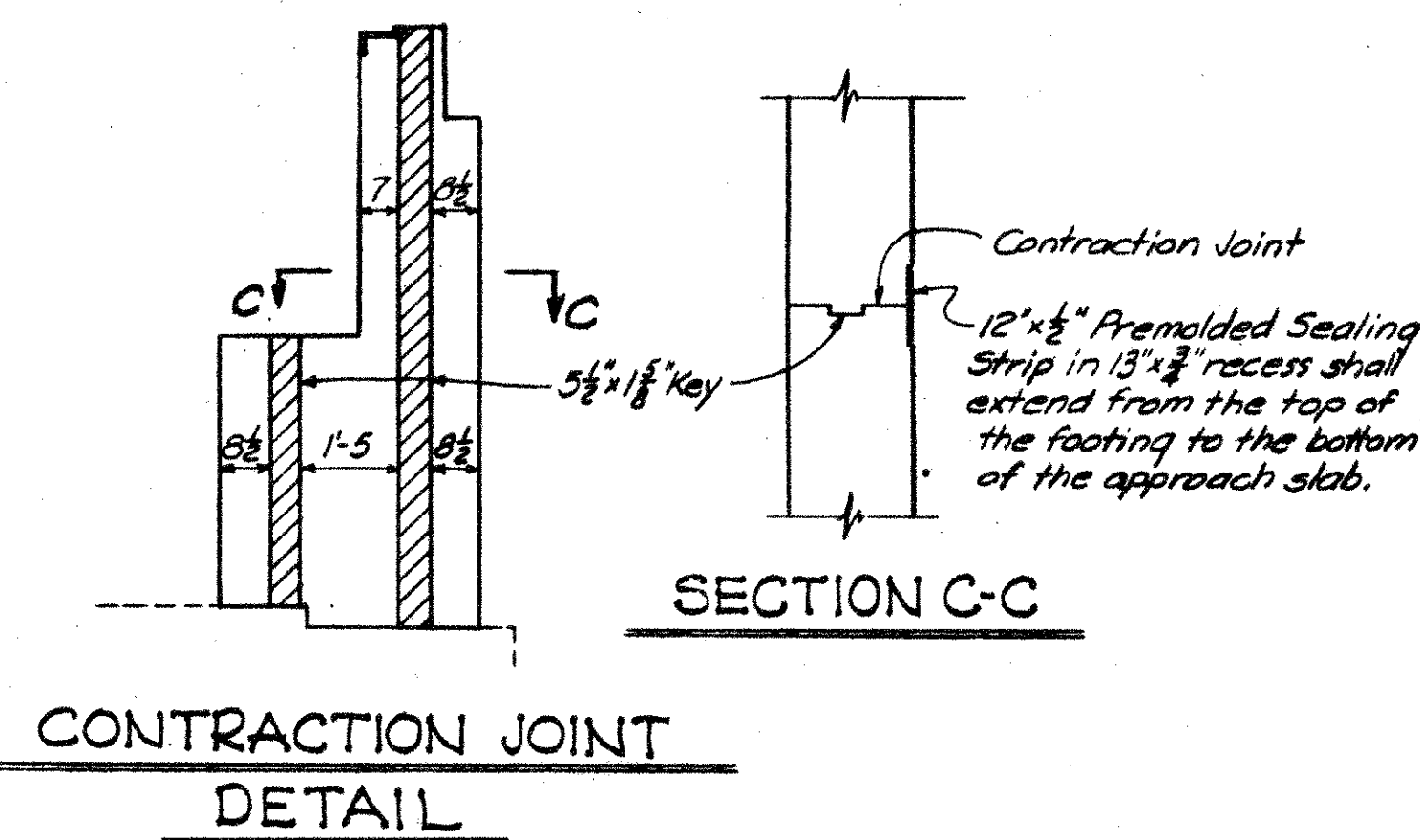
SECTION A-A



SECTION F-F

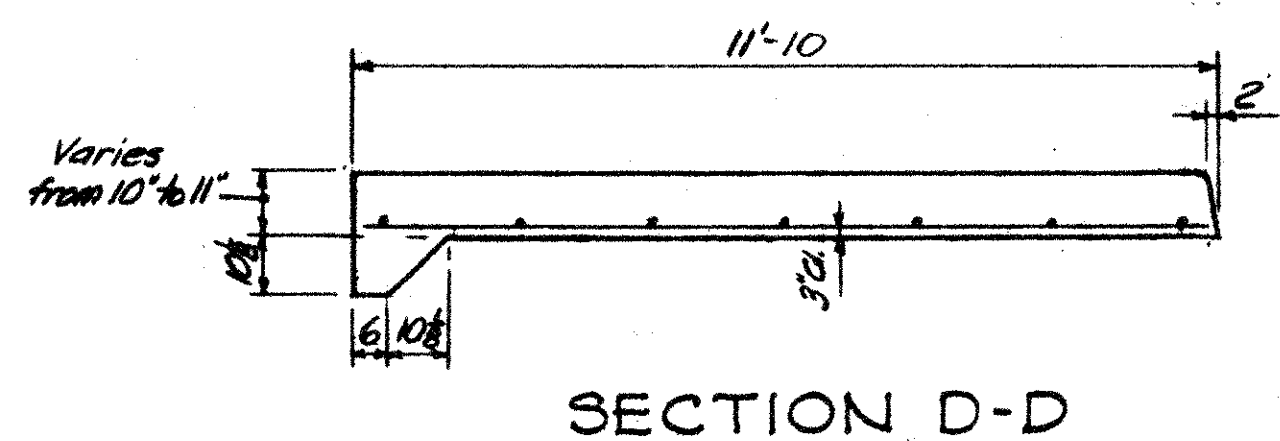


APPROACH SIDEWALK DETAIL



SECTION C-C

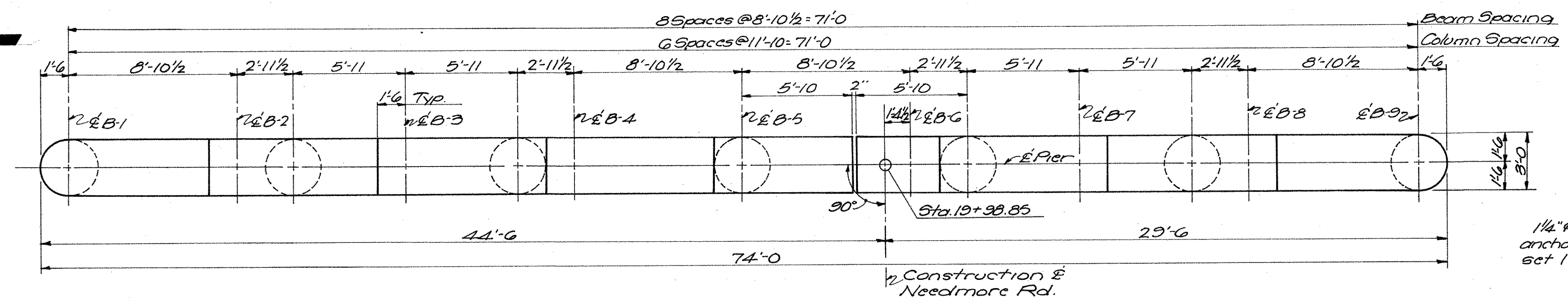
CONTRACTION JOINT DETAIL



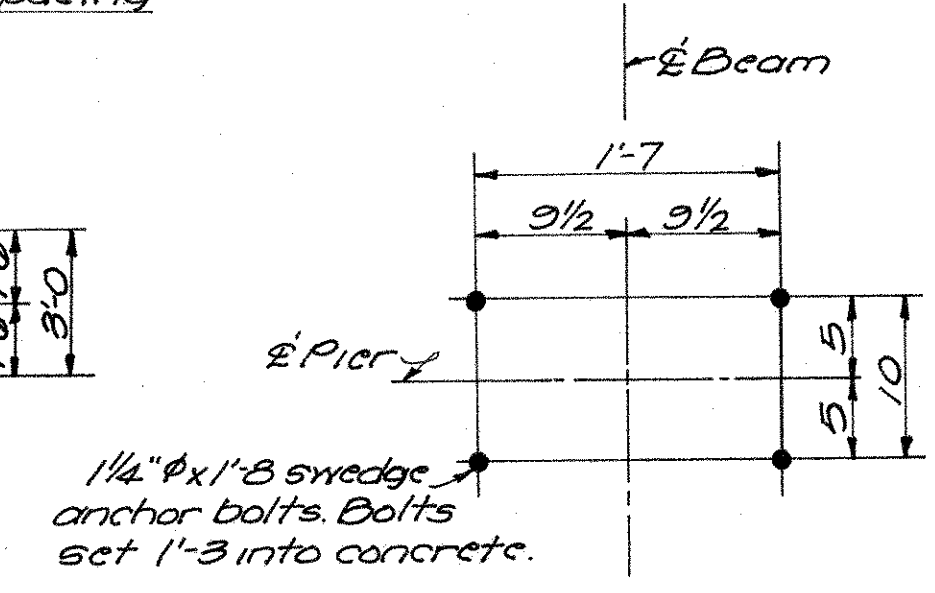
SECTION D-D

- NOTES:
1. For location of Wingwalls, see sheets 3 & 4
 2. For end post details, see Std. Dwg BR-1-65, sheet 2.

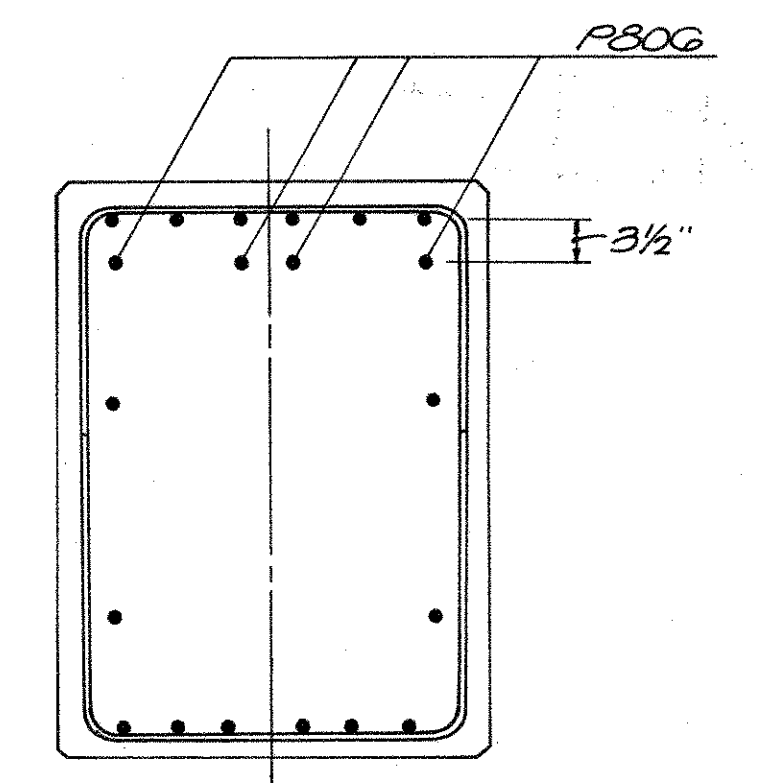
SHAW-LENZ & ASSOCIATES ENGINEERS					5/12
CINCINNATI					OHIO
WINGWALLS & DETAILS					
BRIDGE NO. LOT. 75-1887 NEEDMORE RD OVER I-75					
MONTGOMERY CO.					I-75 STA. 135+65.66
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	R.R.L.	-	R. J. L. 6-4-66		



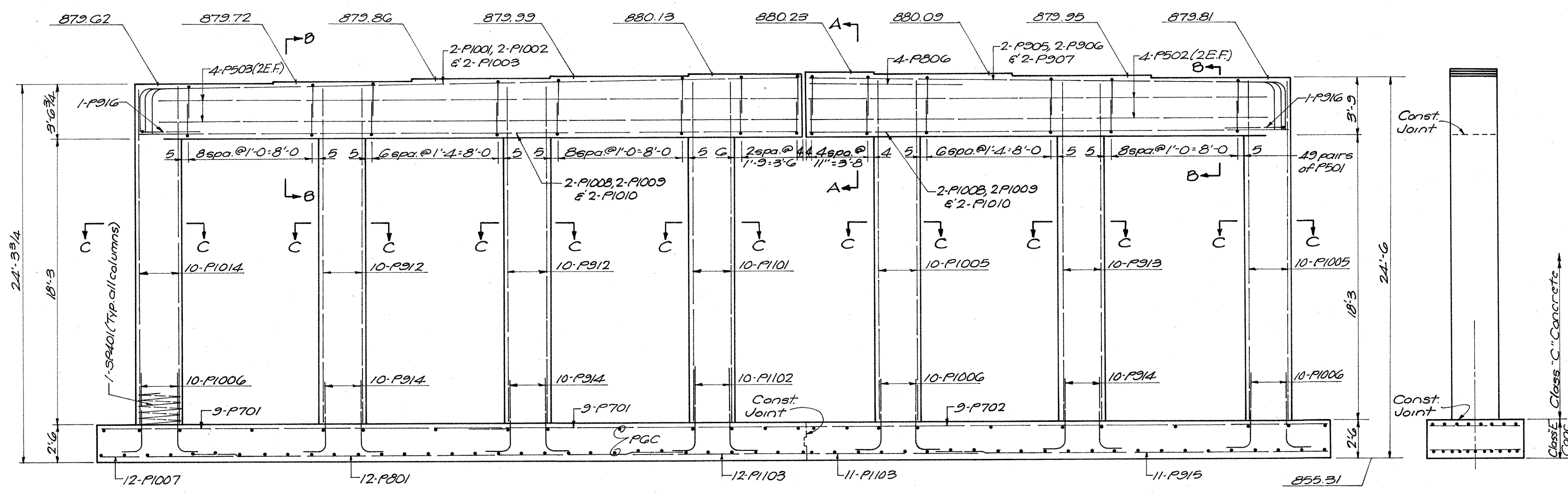
CAP PLAN



ANCHOR BOLT LAYOUT

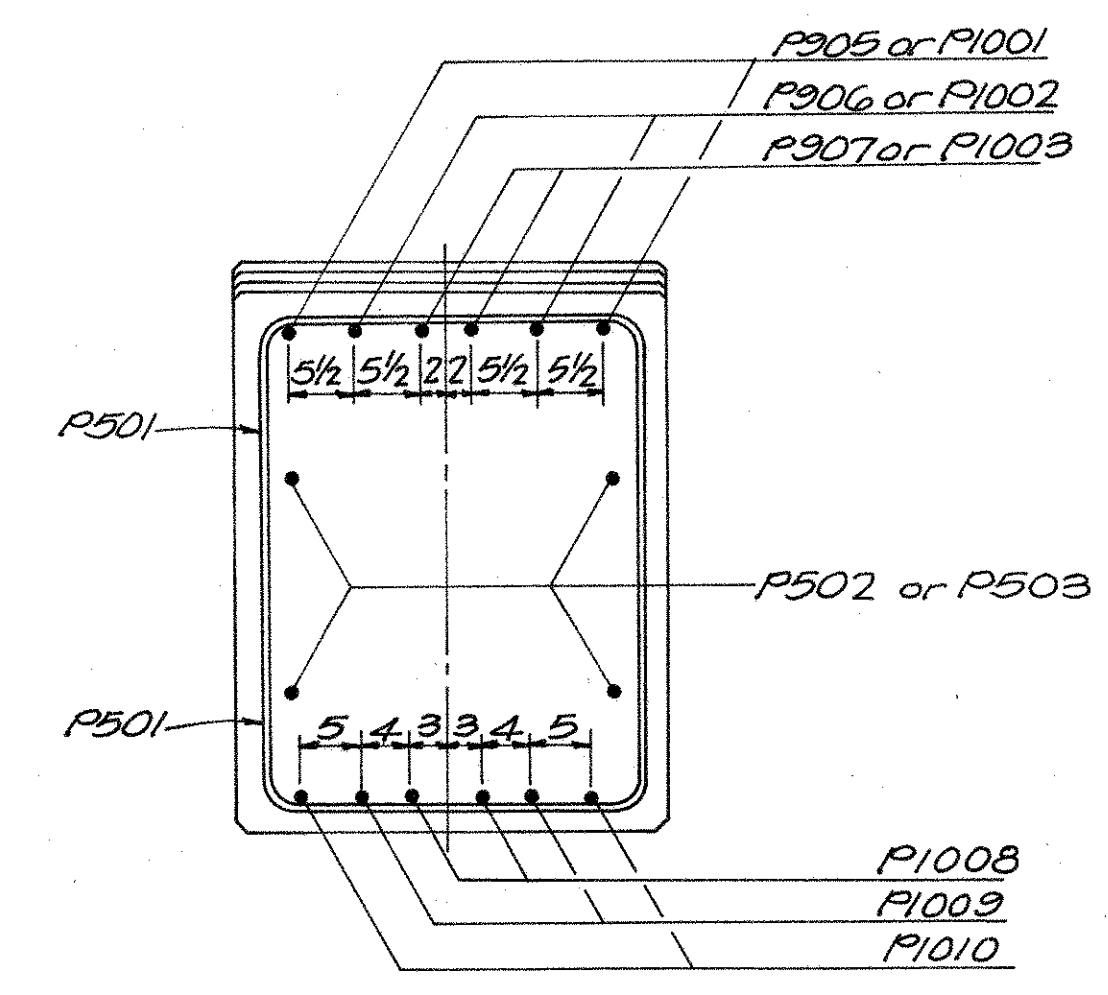


SECTION A-A

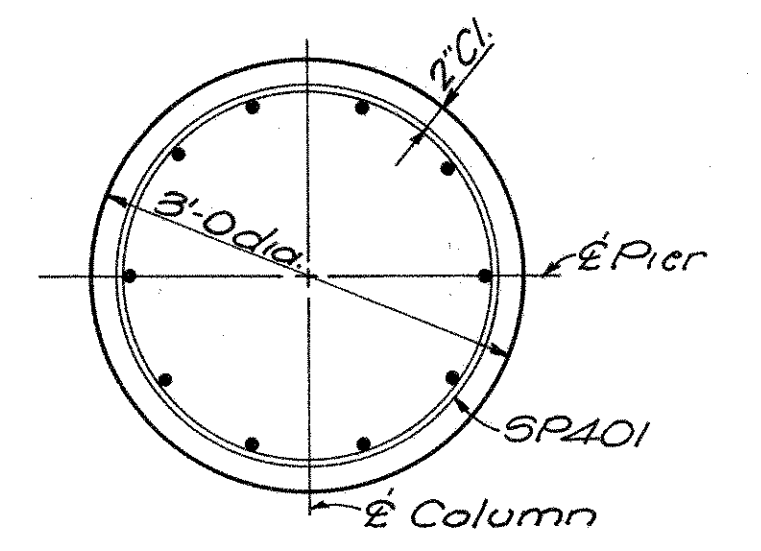


ELEVATION

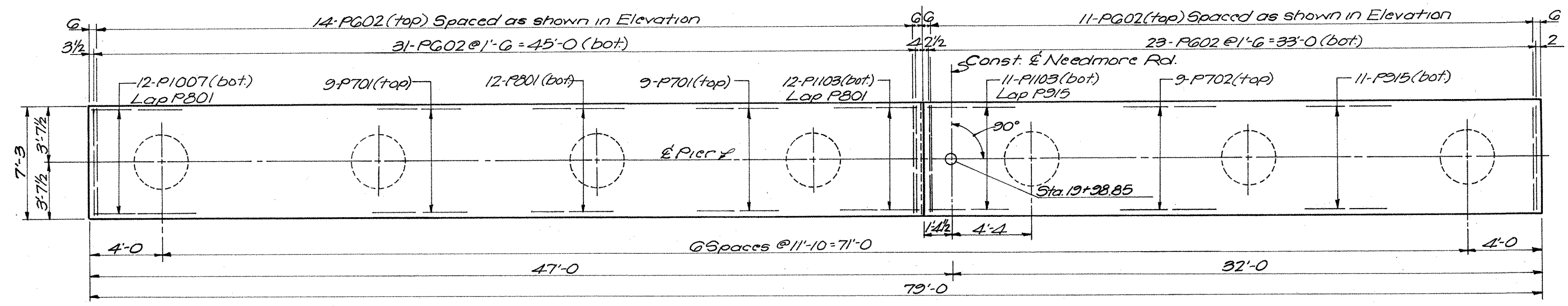
END VIEW



SECTION B-B

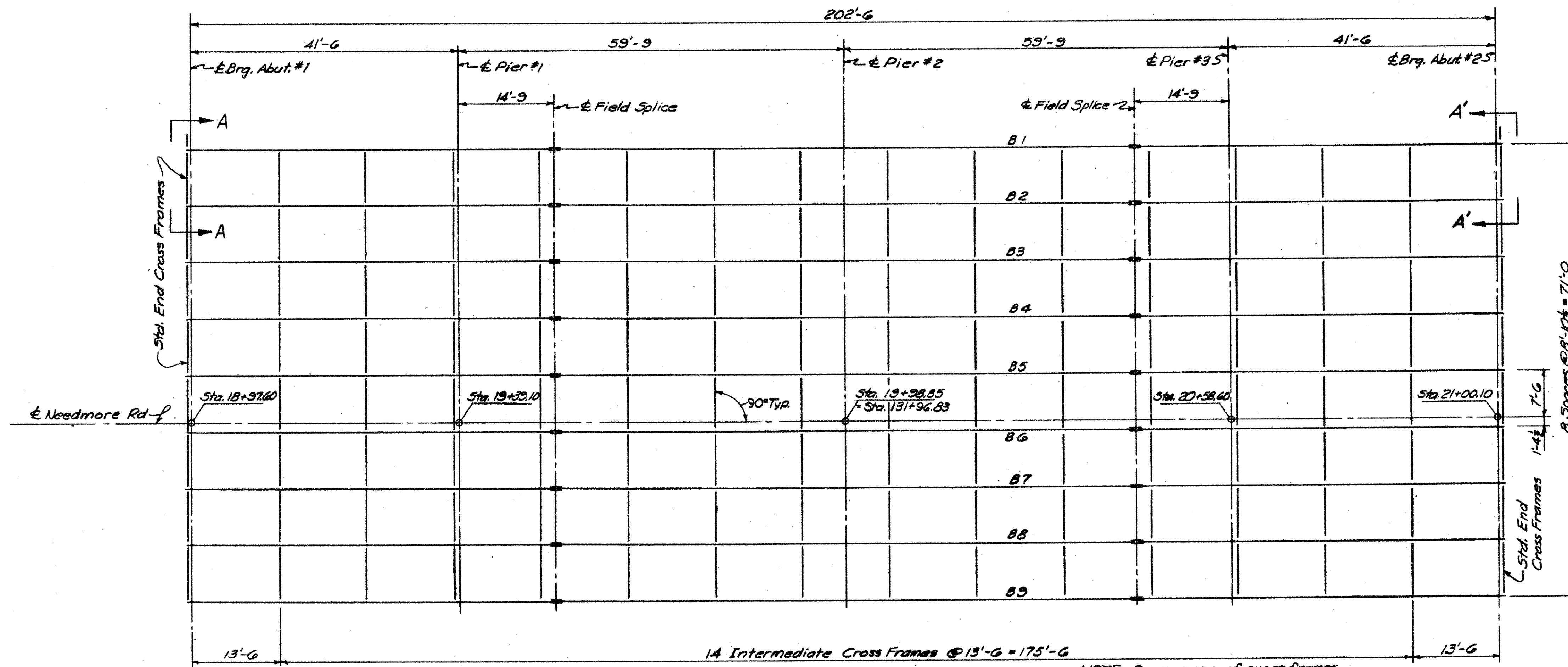


SECTION C-C



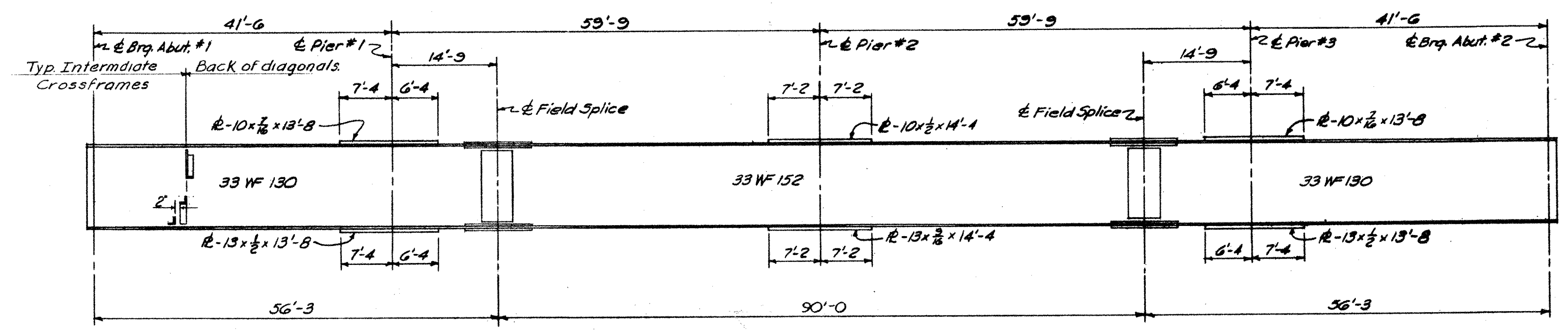
FOOTING PLAN

SHAW, LENZ & ASSOCIATES ENGINEERS				7/12
CINCINNATI				OHIO
PIER #2				
BRIDGE NO. MOT. 75-1887				
NEEDMORE ROAD OVER I-75				
MONTGOMERY CO. STA. 135+65.66				1-75
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
W.B.S.	R.R.L.	W.T.R.	R.J.L.	8-4-66



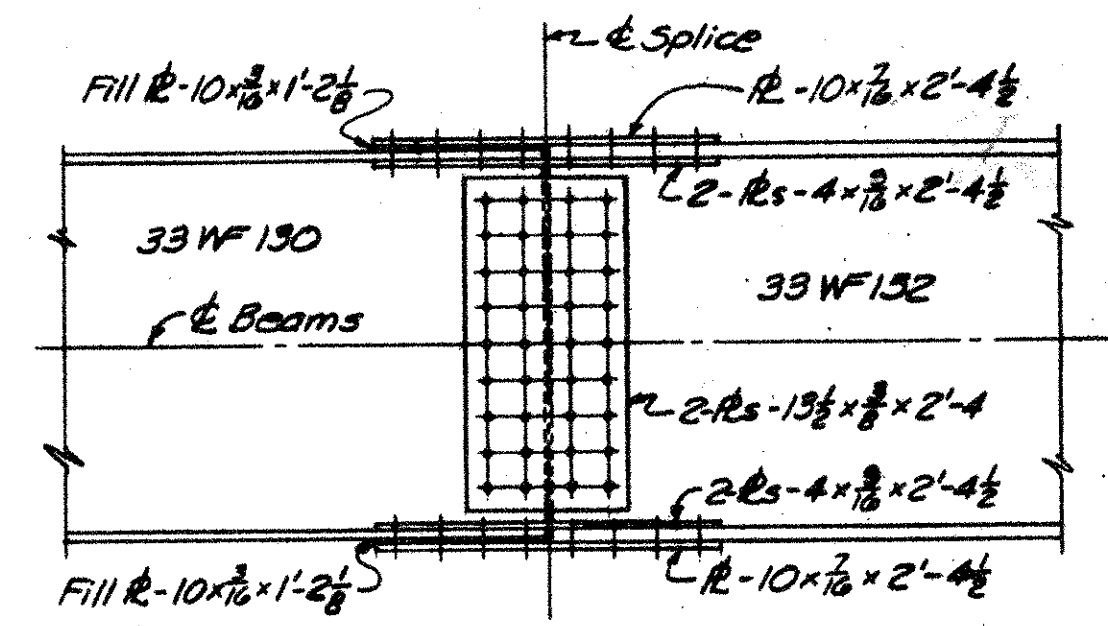
FRAMING PLAN

NOTE: Dimensions of cross frames are to back of diagonals.



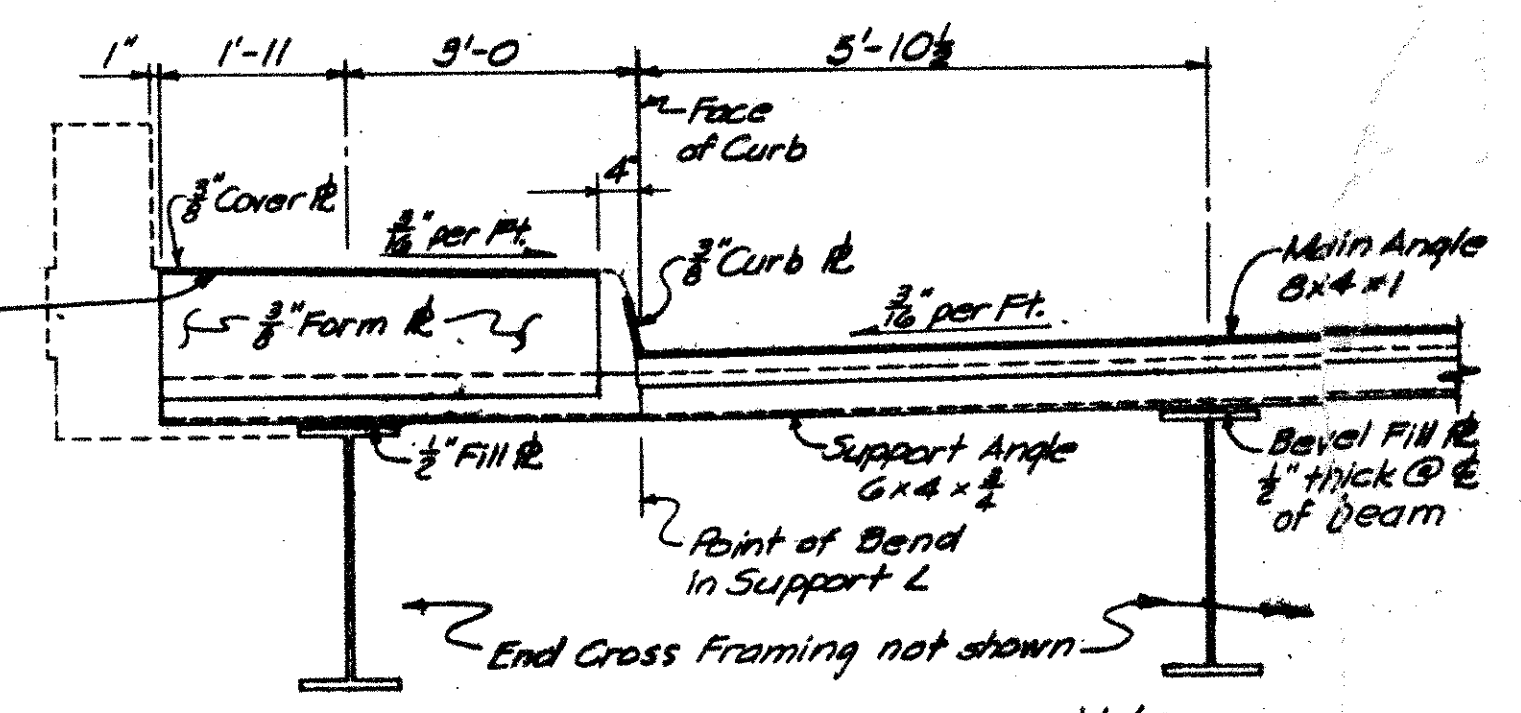
TYPICAL BEAM ELEVATION

NOTES:
 For location of scuppers see sheet
 For scupper details see Std. Dwg. SD-1-65 sheet 2
 For end cross frames, roadway end dam, sidewalk end dam, curb plate details see Std. Dwg. SD-1-65, sheets 1 & 2.
 For details of rockers & bolsters see Std. Dwg. DB-1-55. Omit keeper plates from rockers and bolsters carrying the fascia beams.
 For intermediate cross frame details see sheet
 For shop welding of moment plates see Std. Dwg. SD-1-65, sheet 2.



SPlice DETAIL

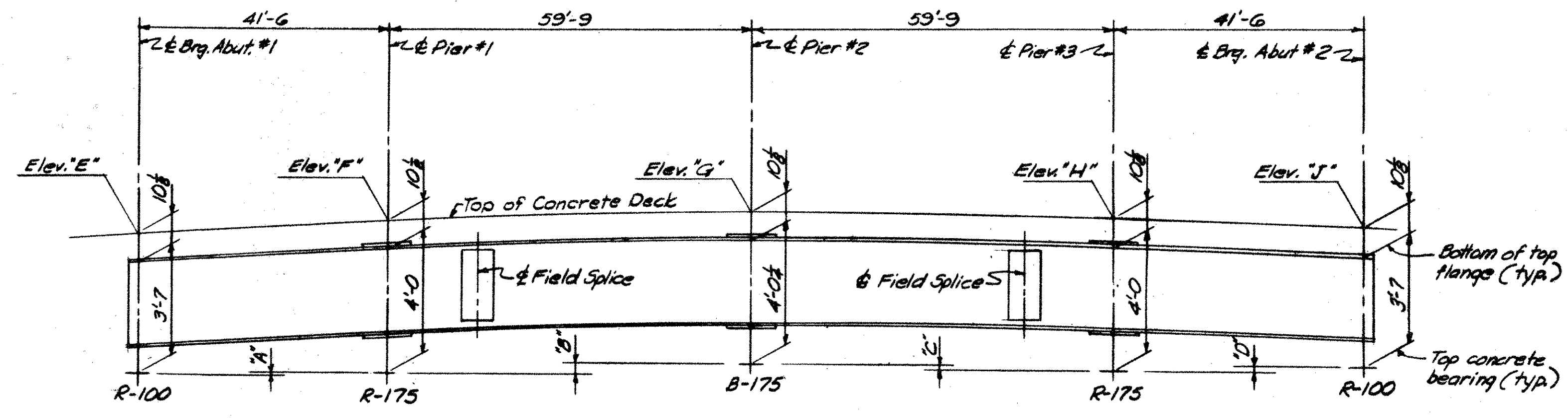
For additional details and dimensions, see Std. Dwg. SD-1-65, sheet 3.



SECTION A-A
(Section A'-A' similar)

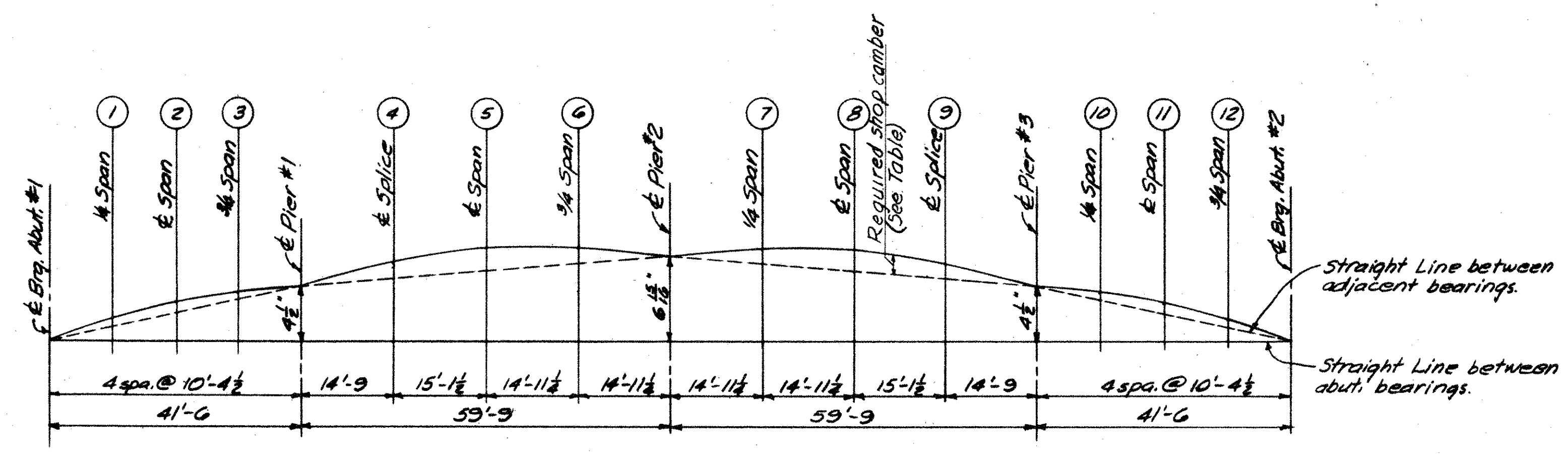
Note:
 Grade @ Abut. #1 = +1.26 %
 Grade @ Abut. #2 = -1.04 %

SHAW, LENZ & ASSOCIATES ENGINEERS					8/12
CINCINNATI					OHIO
FRAMING PLAN					
BRIDGE NO. MOT-75-1887					
NEEDMORE RD. OVER I-75					
MONTGOMERY CO. I-75 STA. 135+65.66					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISI
W.B.S.	R.R.L.	-	R.J.L.		
			8-4-66		



ELEVATION LAYOUT

BEAM	Dim. "A"	Dim. "B"	Dim. "C"	Dim. "D"	Elev. "E"	Elev. "F"	Elev. "G"	Elev. "H"	Elev. "J"
B-1	1/8"	2 3/8"	1 5/16"	1 1/16"	883.80	884.22	884.49	884.36	884.03
B-2	1/8"	3"	1 7/16"	1 1/16"	883.89	884.31	884.58	884.45	884.12
B-3	1/8"	3"	1 7/16"	1 1/16"	884.03	884.45	884.72	884.59	884.26
B-4	1/8"	2 3/8"	1 5/16"	1 1/16"	884.17	884.59	884.86	884.73	884.40
B-5	1/8"	2 3/8"	1 7/16"	1 1/16"	884.31	884.73	885.00	884.86	884.53
B-6	1/8"	3"	1 5/16"	1 5/16"	884.40	884.82	885.09	884.96	884.63
B-7	0"	3"	1 5/16"	1 5/16"	884.26	884.69	884.95	884.82	884.49
B-8	0"	3"	1 5/16"	1 1/16"	884.12	884.55	884.82	884.68	884.35
B-9	0"	3"	1 5/16"	1 1/16"	883.99	884.41	884.68	884.54	884.21



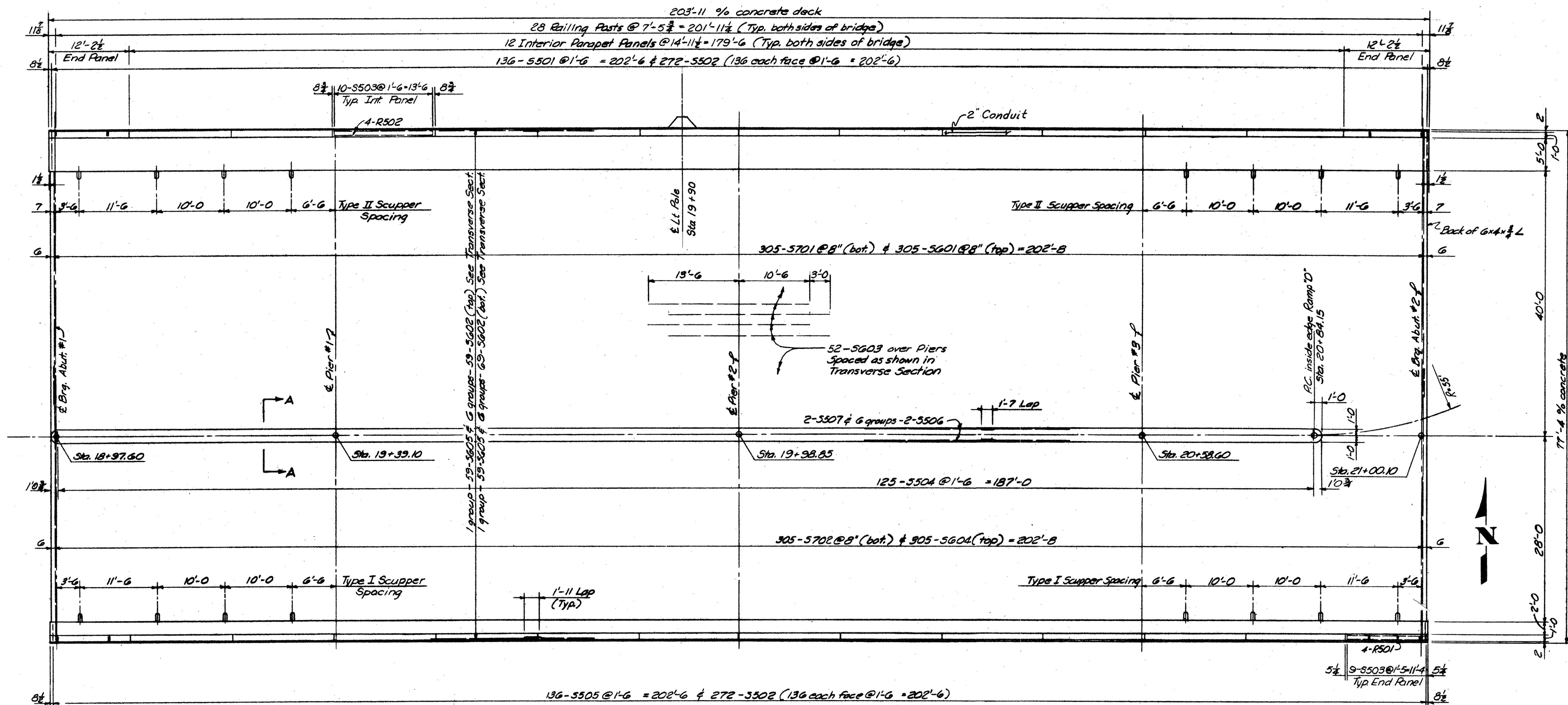
DEFLECTION & CAMBER DIAGRAM

Beam	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)
B-1	Deflection due to D.L. Beam	0	0	0	1/32	1/32	1/32	1/32	1/32	1/32	0	0
	Deflection due to remaining D.L.	3/32	3/32	1/32	5/32	9/32	5/32	5/32	9/32	5/32	1/32	3/32
	Vertical Curve Adjustment	7/32	9/32	7/32	7/16	19/32	7/16	7/16	19/32	7/16	1/32	9/32
	Shop Camber Req'd.	5/16	3/8	1/4	5/8	15/16	5/8	5/8	13/16	5/8	1/4	3/8
B-2 thru B-9	Deflection due to D.L. Beam	0	0	0	1/32	1/32	1/32	1/32	1/32	1/32	0	0
	Deflection due to remaining D.L.	3/32	3/32	1/32	5/32	1/4	1/8	1/8	1/4	5/32	1/32	3/32
	Vertical Curve Adjustment	7/32	9/32	7/32	7/16	19/32	7/16	7/16	19/32	7/16	1/32	9/32
	Shop Camber Req'd.	5/16	3/8	1/4	5/8	7/8	5/8	5/8	7/8	5/8	1/4	3/8

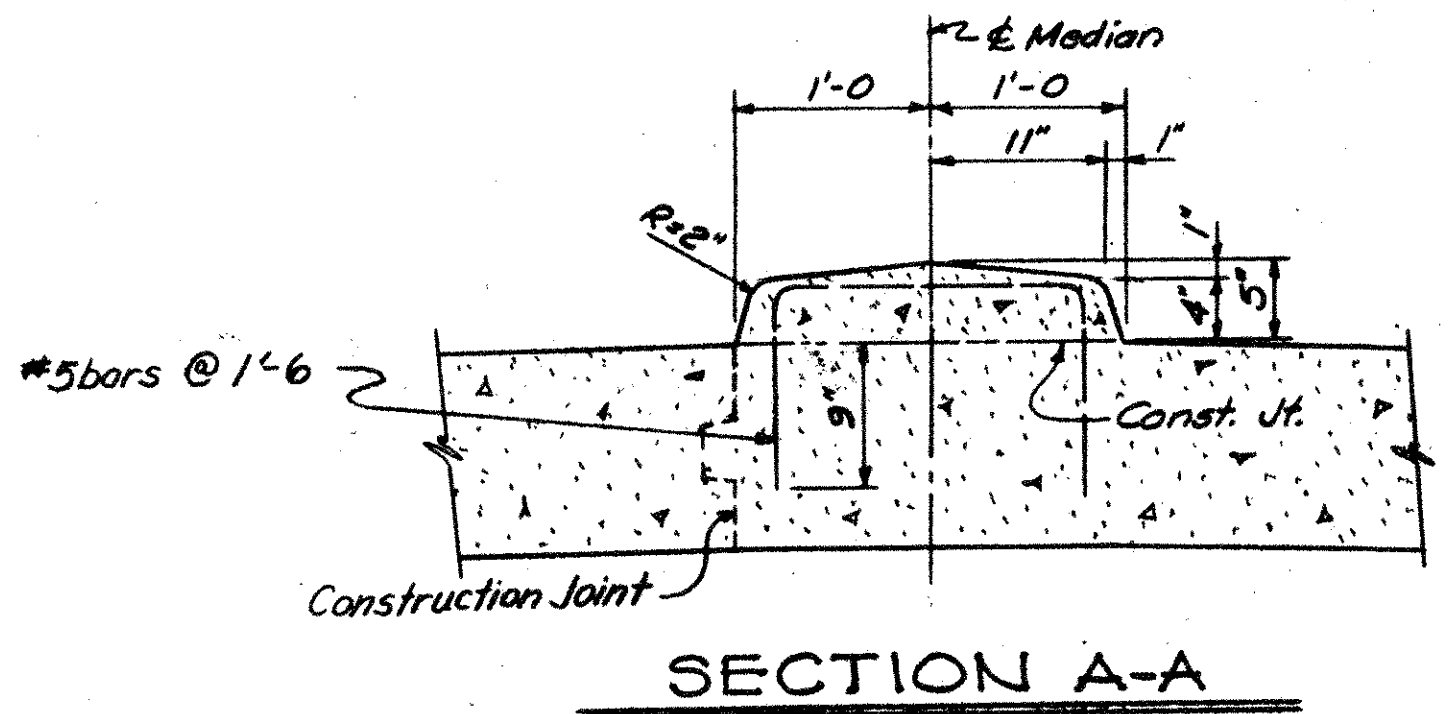
SHAW, LENZ & ASSOCIATES ENGINEERS
CINCINNATI OHIO 9/12

STEEL DETAILS
BRIDGE NO. MOT.-75-187
NEEDMORE ROAD OVER I-75

MONTGOMERY CO. STA. 135+65.66
DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
W.B.S. R.P.L. R.J.L. 8-4-66



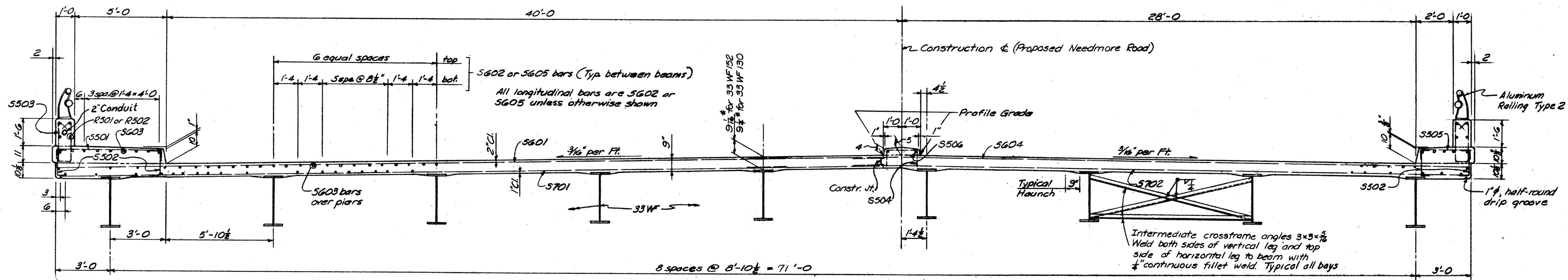
PLAN



SECTION A-A

- NOTES:**
1. Slab thickness includes 1" monolithic wearing surface.
 2. Spread or cut longitudinal reinforcing steel in slab to clear scuppers.
 3. For scupper details, see Std. Dwg. SD-1-65, sheet 2.
 4. For end crossframe & end dam details, see Std. Dwg. SD-1-65, sheet 1.
 5. For aluminum railing details, see Std. Dwg. BR-1-65, sheet 2.
 6. For sidewalk end dam details & curb plate details, see Std. Dwg. BR-1-65, sheet 2.
 7. For pavement screed elevations, see sheet
 8. For railing and parapet joint details, see Std. Dwg. BR-1-65, Sheet 1.
 9. For light pole support details see sheet 373.

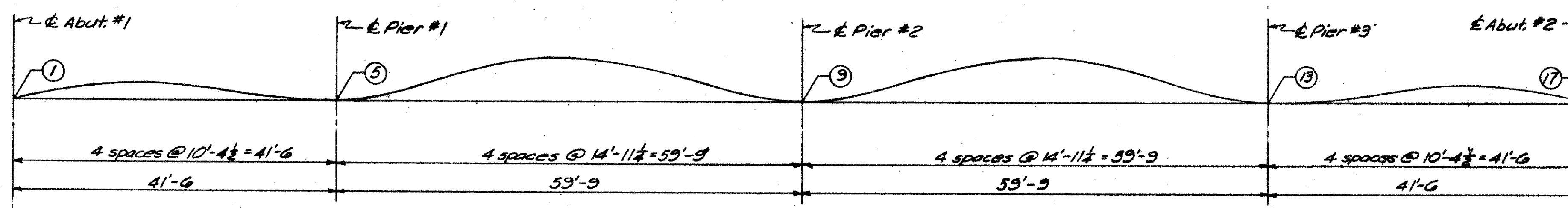
SHAW, LENZ & ASSOCIATES ENGINEERS						10/12
CINCINNATI						OHIO
SUPERSTRUCTURE SLAB						
BRIDGE NO. MOT-75-1887 NEEDMORE ROAD OVER I-75						
I-75						
MONTGOMERY CO.						STA. 19+65.66
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED	
W.B.S.	R.R.L.	-	R.J.L. 8-4-66			



TRANSVERSE SECTION

DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of the steel beams, which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12", except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width.

* This is a nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade.

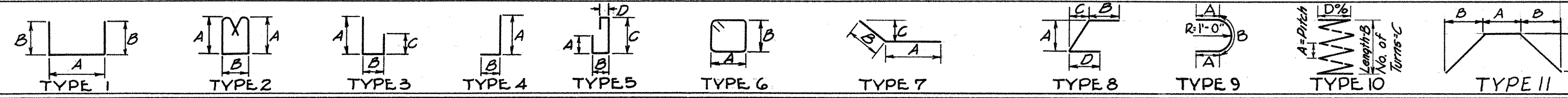


SCREED ELEVATION DIAGRAM

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17
Station	18+97.60	19+07.98	19+18.35	19+28.72	19+39.10	19+54.04	19+68.98	19+83.91	19+98.85	20+13.79	20+28.72	20+43.66	20+58.60	20+68.98	20+79.35	20+89.72	21+00.10
Profile Grade	884.408	884.532	884.643	884.743	884.830	884.935	885.015	885.069	885.099	885.103	885.082	885.036	884.965	884.901	884.825	884.737	884.636
Gutter Elev. 28' Rt.	883.986	884.110	884.221	884.321	884.409	884.513	884.593	884.648	884.677	884.681	884.660	884.615	884.544	884.479	884.403	884.315	884.215
Concrete D.L. Defl. Rt. Gutter	0.000	0.007	0.008	0.002	0.000	0.013	0.021	0.011	0.000	0.011	0.021	0.013	0.000	0.002	0.008	0.007	0.000
Screed Setting Elev. Rt. Gutter	883.99	884.12	884.23	884.32	884.41	884.53	884.61	884.66	884.68	884.69	884.68	884.63	884.54	884.48	884.41	884.32	884.22
Gutter Elev. 40' Lt.	883.798	883.922	884.034	884.134	884.221	884.326	884.406	884.460	884.489	884.494	884.473	884.427	884.356	884.292	884.216	884.128	884.027
Concrete D.L. Defl. Lt. Gutter	0.000	0.008	0.008	0.002	0.000	0.013	0.022	0.012	0.000	0.012	0.022	0.013	0.000	0.002	0.008	0.008	0.000
Screed Setting Elev. Lt. Gutter	883.80	883.93	884.04	884.14	884.22	884.34	884.43	884.47	884.49	884.51	884.50	884.44	884.36	884.29	884.22	884.14	884.03

SHAW, LENZ & ASSOCIATES		11/12	
ENGINEERS		OHIO	
CINCINNATI			
SUPERSTRUCTURE SLAB			
BRIDGE NO. MOT-75-1887			
NEEDMORE ROAD OVER I-75			
I-75			
MONTGOMERY CO.		STA. 131+65.66	
DESIGNED	DRAWN	TRACED	CHECKED
W.B.S.	R.R.L.	-	R.J.L.
			8-4-66
REVIEWED	REVISOR		

REINFORCING STEEL BAR SCHEDULE



MARK	NO.	SIZE	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
SUPERSTRUCTURE										
5701	305		44'-8"	Str.					27,846	Slab
5702	305		34'-3"	Str.					21,352	"
5601	305		44'-8"	Str.					20,462	Slab
5602	768		30'-0"	↑					34,606	"
5603	156		24'-0"	↓					5,623	Slab over Piers
5604	305		33'-11"	↓					15,538	Slab
5605	59		35'-1"	Str.					3,109	"
5501	136		6'-7"	1	5'-6"	0'-8"			934	Sidewalk
5502	544		2'-5"	1	1'-4"	0'-8"			1,371	Curb & Sidewalk
5503	276		5'-7"	2	2'-2"	0'-8"			1,607	"
5504	125		2'-10"	1	1'-7"	0'-9"			369	Slab under Median
5505	136		3'-7"	1	2'-6"	0'-8"			508	Curb
5506	12		30'-0"	Str.					375	Median Curb
5507	2		17'-9"	"					37	"
L601	5		7'-0"	11	1'-3"	2'-1"	2'-1"		53	Light Pole
* R501	16		11'-11"	Str.						End Parapet Panels
* R502	96		14'-8"	Str.						Int. Parapet Panels
L602	4		10'-4"	6	2'-1"	2'-10"			62	Light Pole
ABUTMENTS										
A801	28		40'-5"	Str.					3,022	Ftgs.
A802	24		10'-3"	"					657	"
AG01	100		14'-3"	3	6'-7"	5'-5"	2'-7"		2,140	Ftgs.
AG02	146		14'-4"	5	4'-8"	1'-5"	6'-0"	0'-11"	3,143	Backwall
AG03	16		18'-2"	1	1'-2"	8'-8"			437	Wingwalls
AG04	8		14'-0"	1	1'-2"	6'-7"			168	"
A501	28		6'-6"	Str.					190	Abut. Corners
A502	100		7'-2"	1	5'-5"	1'-0"			747	Ftgs.
A503	100		7'-5"	4	6'-7"	1'-0"			774	Seats & Ftgs.
A504	34		44'-6"	Str.					1578	Seats & Backwalls
A505	34		31'-6"	Str.					1117	"
A506	100		5'-2"	1	3'-5"	1'-0"			539	Seats
A508	40		5'-7"	2	2'-2"	0'-8"			233	Curbs
A509	32		12'-3"	6	3'-2"	2'-7"			409	Ftgs.
A510	24		7'-3"	Str.					181	Wingwalls
A511	8		11'-10"	7	7'-5"	4'-6"	2'-0"		99	"
A512	8		7'-11"	Str.					66	"
A513	32		12'-9"	Str.					426	"
A514	12		5'-2"	Str.					65	Wingwalls #2 & #4
A515	4		5'-8"	Str.					24	"
A516	2		11'-1"	1	1'-2"	5'-1"			23	"
A517	2		9'-7"	1	1'-2"	4'-4"			20	"
A518	6		8'-1"	1	1'-2"	3'-7"			51	"
A519	14		3'-4"	8	1'-4"	1'-0"	0'-2"	1'-2"	49	Curbs, Wingwalls #2 & #4
A520	6		6'-1"	1		2'-6"			38	"
A521	2		5'-9"	1		2'-4"			12	"
A522	2		5'-1"	1		2'-0"			11	"
A523	2		4'-1"	1		1'-6"			9	"
A524	2		3'-1"	1	1'-4"	1'-0"			6	"
A525	6		8'-2"	3	5'-2"	0'-6"	2'-9"		51	Wingwalls #1/4 #3
A526	6		3'-3"	Str.					20	"
A527	2		8'-3"	3	5'-8"	0'-6"	2'-9"		18	"
A528	2		3'-9"	Str.					8	"
A529	2		12'-11"	5	3'-4"	1'-2"	5'-2"	0'-6"	27	"
A530	2		11'-4"	5	2'-7"	1'-2"	4'-4"	0'-6"	24	"
A531	6		9'-10"	5	1'-10"	1'-2"	3'-7"	0'-6"	62	"

MARK	NO.	SIZE	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
* R503	12									See Std. Dwg BR-1-65
* R504	8									Sheet 2 for details
* R505	8									"
* R506	16		12'-5"	Str.						"
PIERS										
P1101	30		22'-0"	Str.						3507
P1102	30		7'-2"	4	5'-8"	1'-9"				1142
P1103	65		11'-0"	Str.						3799
P1001	2		44'-5"	4	41'-5"	3'-3"				382
P1002	2		45'-1"	4	42'-1"	3'-3"				388
P1003	2		45'-4"	4	42'-4"	3'-3"				390
P1005	40		21'-9"	Str.						3744
P1006	70		6'-6"	4	5'-4"	1'-5"				1958
P1007	34		8'-6"	Str.						1244
P1008	8		30'-7"	↑						1053
P1009	8		30'-6"	↓						1050
P1010	8		29'-3"	↓						1007
P1014	30		21'-6"	↓						2775
P1015	4		42'-5"	↓						730
P1016	4		42'-4"	↓						729
P1017	4		41'-1"	Str.						707
P901	4		44'-6"	4	41'-5"	3'-3"				605
P902	4		45'-2"	4	42'-1"	3'-3"				614
P903	4		45'-5"	4	42'-4"	3'-3"				618
P904	2		45'-4"	4	42'-3"	3'-3"				308
P905	2		32'-10"	4	29'-9"	3'-3"				223
P906	2		33'-6"	4	30'-5"	3'-3"				228
P907	2		33'-9"	4	30'-8"	3'-3"				230
P912	60		21'-7"	Str.						4403
P913	50		21'-10"	Str.						3712
P914	110		5'-8"	4	4'-11"	0'-11"				2119
P915	31		24'-11"	Str.						2626
P916	6		8'-10"	9	2'-10"	3'-2"				180
P801	34		30'-10"	Str.						2799
P802	4		32'-10"	4	29'-9"	3'-3"				351
P803	4		33'-6"	4	30'-5"	3'-3"				358
P804	4		33'-9"	4	30'-8"	3'-3"				360
P805	2		33'-8"	4	30'-7"	3'-3"				180
P806	12		10'-0"	Str.						320
P701	50		24'-0"	Str.						2453
P702	25		33'-0"	Str.						1686
P601	130		5'-11"	Str.						1155
P602	79		6'-11"	Str.						821
P501	294		7'-9"	1	2'-8"	2'-8"				2376
P502	12		29'-4"	Str.						367
P503	12		41'-2"	Str.						515
SP401	21		18'-3"	10	4'-6"	18'-3"	51	2'-8"		6972

NOTES:

- All dimensions are out to out of bar.
- Radius dimensions are to outside of bar.
- The length of bent bars is measured along centerline.
- Bar size is indicated in the bar mark. The first digit where three digits are used the first two digits where four are used indicates the bar size.
- Four steel channel, tee or angle spacers weighing approximately 0.68 lbs. per lin. ft. of spacer shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantity for spiral bars.
The "Length" shown in the steel list for the spiral bars is the distance from the top of the footings to the bottom of the pier cap.
The "No. of Turns" shown is the "Length" divided by the pitch, plus 3 turns (total number of closed coils), expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item 509 1/2 closed coils shall be provided at the ends of each spiral unit.

REPLACEMENT BARS			
MARK	NO.	LENGTH	TYPE
RE401	1	6'-3"	Str.
RE501	1	6'-7"	↑
RE601	3	6'-11"	↑
RE701	3	7'-3"	↑
RE801	1	7'-6"	↑
RE901	1	7'-10"	↑
RE1001	1	8'-3"	↓
RE1101	1	8'-7"	Str.

* Included with Item 517 for payment.

SHAW, LENZ & ASSOCIATES 12/12
ENGINEERS

CINCINNATI OHIO

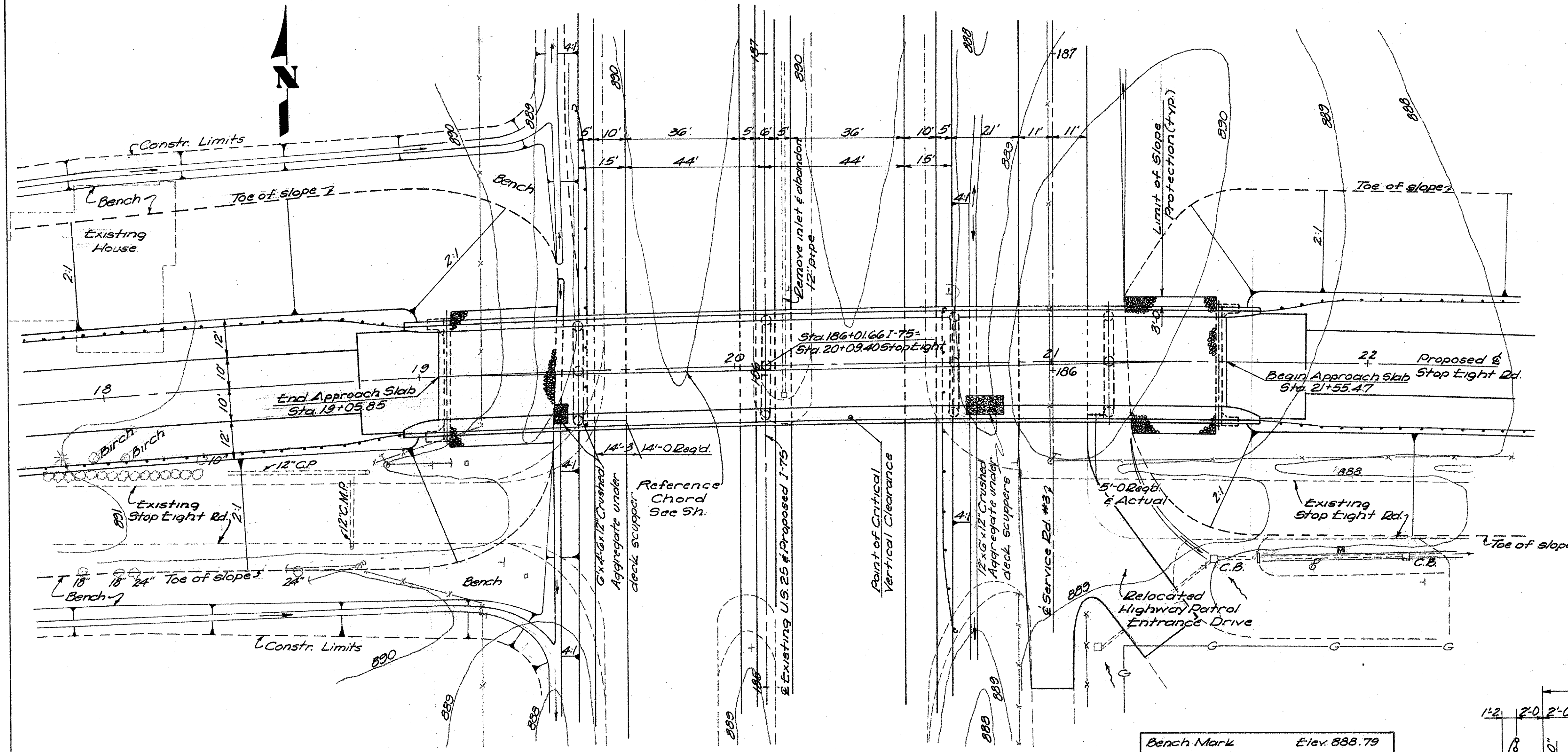
REINFORCING STEEL DETAILS

BRIDGE NO. MOT-75-1887
NEEDMORE ROAD OVER I-75

I-75
STA. 135+63.66

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
R.R.L.	R.R.L.	W.T.R.	R.J.L.		
			8-4-66		

371 Res



PROPOSED STRUCTURE

TYPE: Continuous steel beam with reinforced concrete deck and substructure.
 SPANS: 41'-6", 59'-9", 59'-9", 49'-1", and 35'-0" (measured along chord between abutments)
 ROADWAY: 28'-0" w/ curbs with one 2'-0" safety curb and one 4'-0" sidewalk.
 DESIGN LOAD: AASHTO H520-44
 SKEW: 1°-02'-30" Lt. Forward (chord between abutments)
 WEARING SURFACE: 1" monolithic concrete.
 APPROACH SLAB: 45'-1'-67" (25' long)
 ALIGNMENT: 1°20' Curve to right.

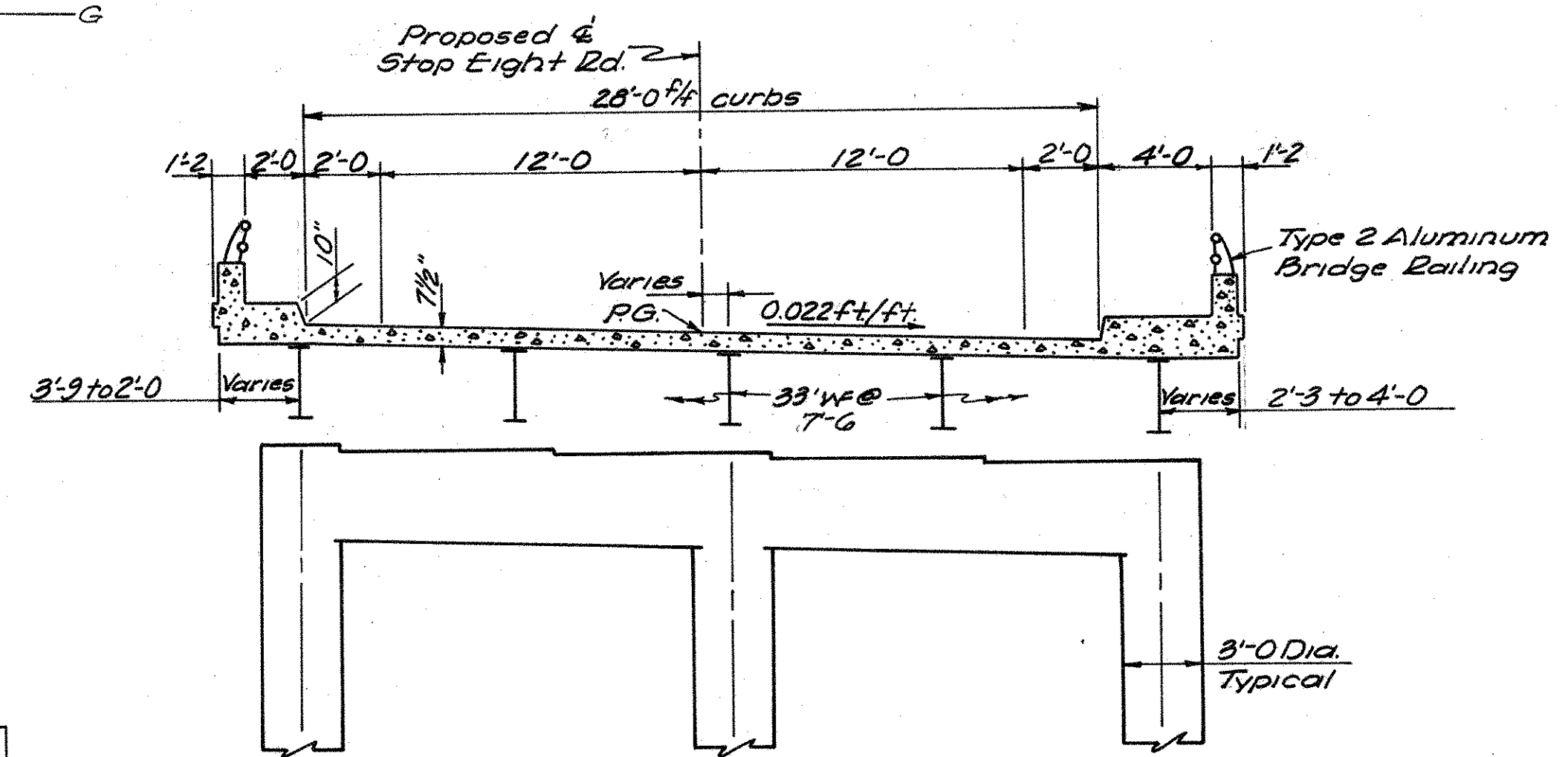
AVERAGE DAILY TRAFFIC

A.D.T. (1975) 960*
 Type "B" (Estimated) ... 48

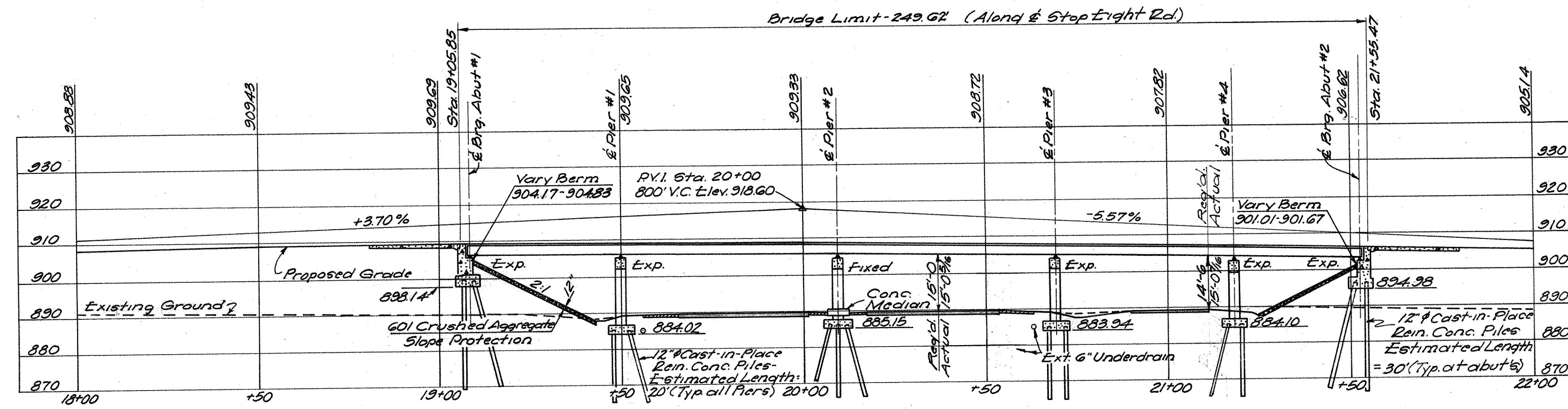
*From Ohio Dept. of Highways Division of Planning and Programming Bureau of Planning Survey - Feb. 1966.

PLAN

Bench Mark Elev. 888.79
 2" cut in top of concrete headwall 64' 12" Sta. 187+64



SECTION THRU BRIDGE DECK



SECTION ALONG & PROPOSED STOP EIGHT ROAD

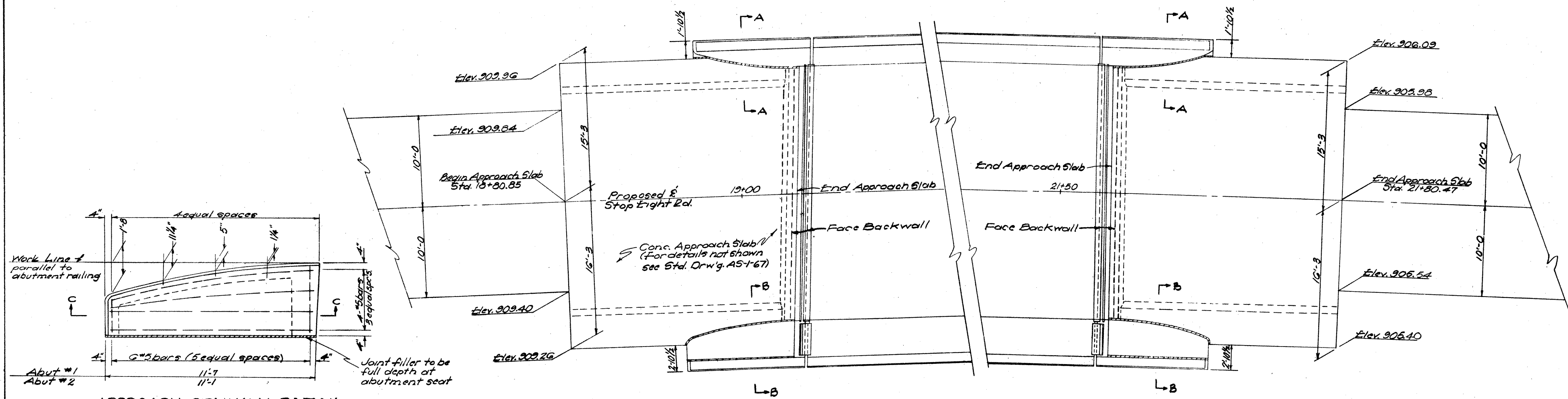
SHAW, LENZ & ASSOCIATES 1/13
 ENGINEERS
 CINCINNATI OHIO

SITE PLAN

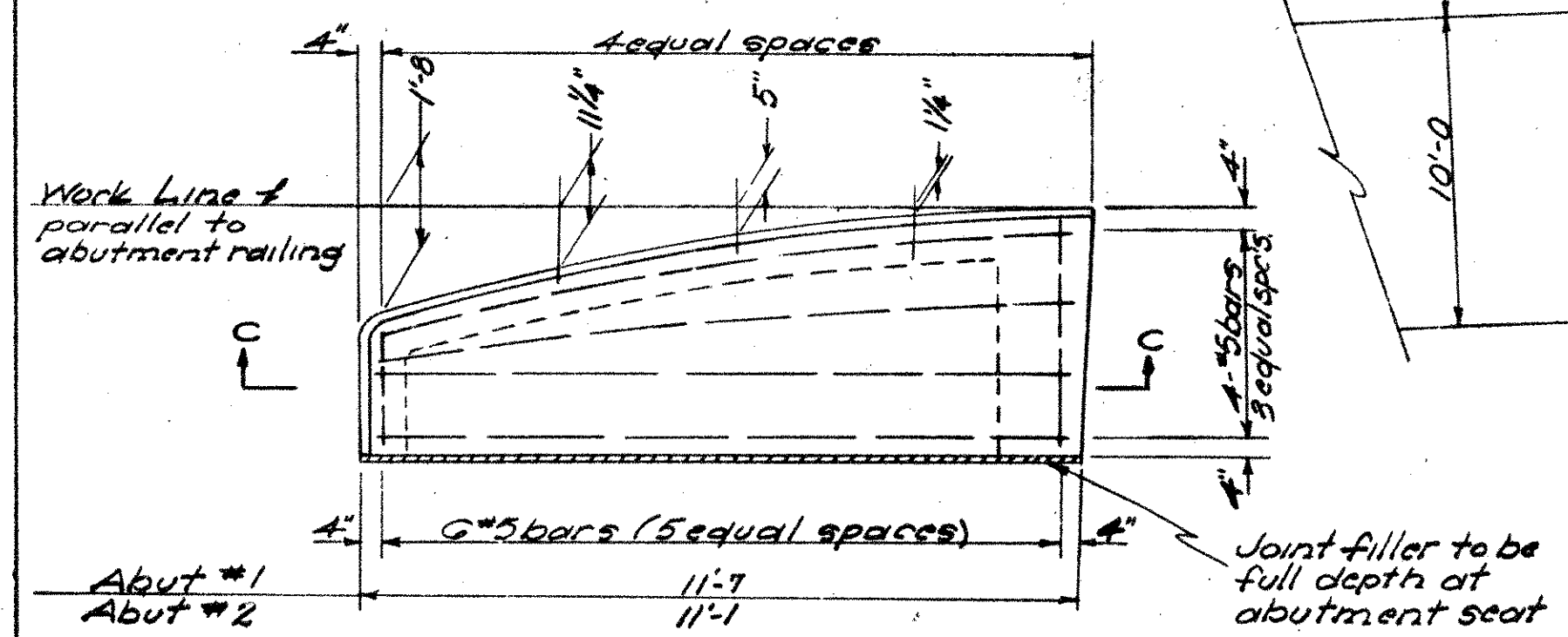
BRIDGE NO. MOT-75-1979
 STOP EIGHT OVER 1-75

MONTGOMERY COUNTY STA. 185+82.49

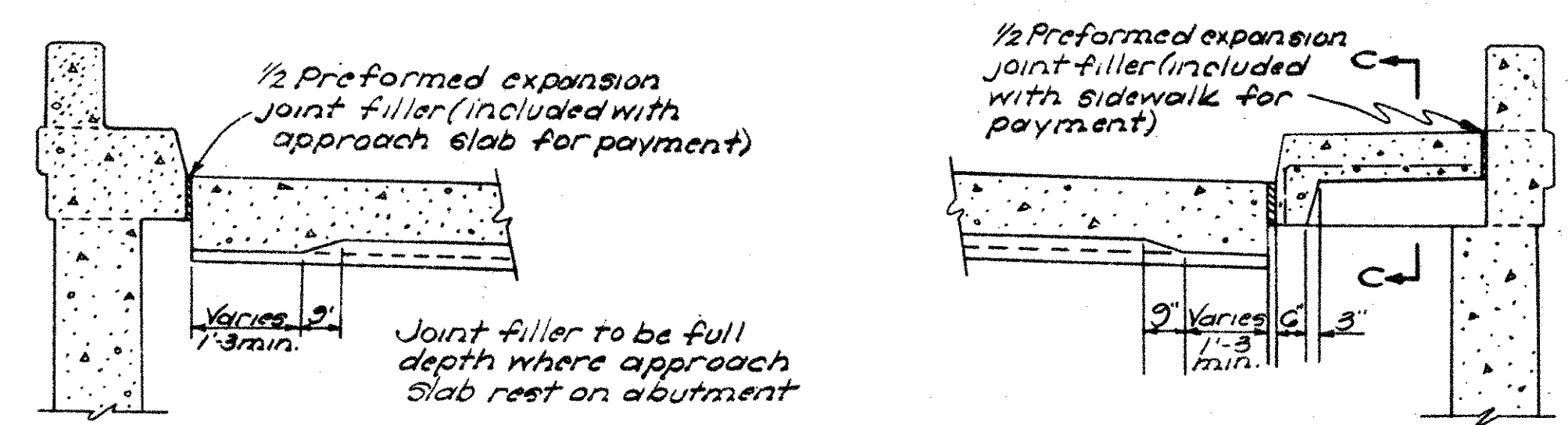
PRESENT TOPO	DESIGNED	DRAWN	CHECKED	REVIEWED
B.H.S.	W.T.R.	W.B.S.	W.T.R.	R.J.L.
				12-29-67



BRIDGE APPROACH PLAN

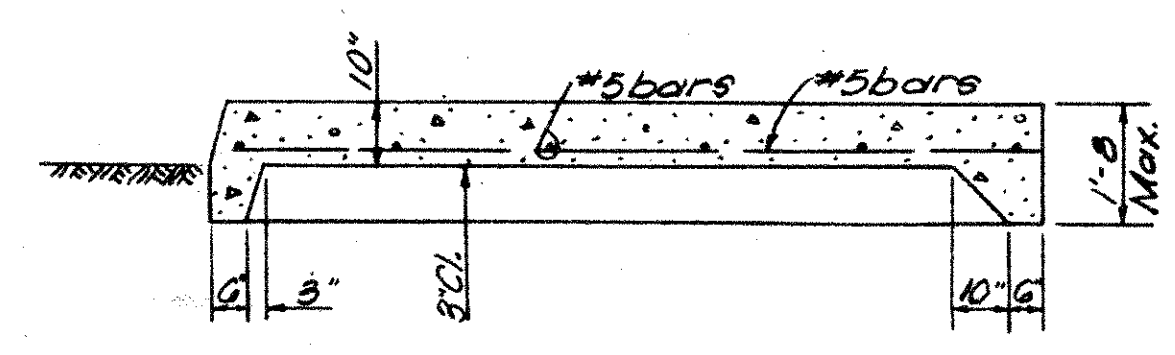


APPROACH SIDEWALK DETAIL
Sidewalk included with roadway quantities

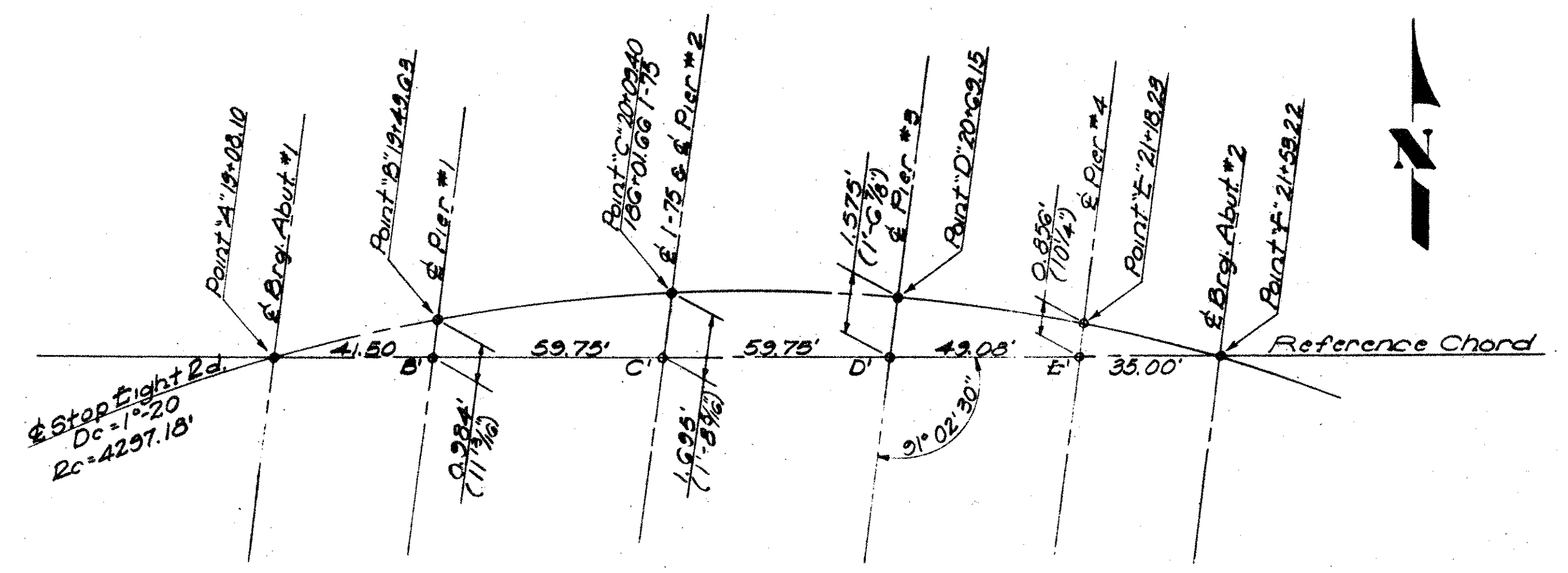


SECTION A-A

SECTION B-B



SECTION C-C



GEOMETRIC LAYOUT
(No Scale)

SHAW, LENZ & ASSOCIATES 2/13				
ENGINEERS OHIO				
CINCINNATI				
GEOMETRIC LAYOUT & APPROACH DETAILS				
BRIDGE NO. MOT. 75-1070				
STOP EIGHT OVER 175				
MONTGOMERY COUNTY STA. 185+82.49				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
W.B.S.	W.B.S.	W.T.R.	R.J.L.	12-29-67

ESTIMATED QUANTITIES							
ITEM	TOTAL	UNIT	DESCRIPTION	ABUTS.	PIERS	SUPER STRUCTURE	GENERAL
503	355	Cu.Yds.	Excavation for structures, unclassified	179	176		
505	Lump	Lump Sum	First Test Pile				Lump
507	1640	Lin.Ft.	12" ϕ Cast-in-Place Reinforced Concrete Piles	600	1040		
509	113,086	Lbs.	Reinforcing Steel	8,409	26,872	77,805	
511	279	Cu.Yds.	Class "C" Concrete - Superstructure			279	
511	91	Cu.Yds.	Class "C" Concrete - Piers above footings		91		
511	78	Cu.Yds.	Class "C" Concrete - Abutments above footings	78			
511	121	Cu.Yds.	Class "C" Concrete - Footings	60	61		
513	189,355	Lbs.	Structural Steel			189,355	
517	542	Lin.Ft.	Bridge Railing, Type 2	49		493	
518	25	Cu.Yds.	Porous Backfill	25			
518	7	Each	Scuppers, Type 2 (including supports)			7	
518	53	Lin.Ft.	6" Perforated Helical C.M.P. including specials 707.01	53			
518	49	Lin.Ft.	6" Non-perforated Helical C.M.P. 707.01	49			
601	336	Sq.Yds.	Crushed Aggregate Slope Protection				336
808	279	Units	Water-reducing, Set-retarding admixture			279	
514	189,355	Lbs.	Field Painting of Structural Steel			189,355	

GENERAL NOTES

REFERENCE shall be made to the following:

Standard Drawings: BR-1-65 sheet 2 revised 11-24-65
 RB-1-55 revised 2-2-59
 SD-1-65 sheets 1, 2 & 3 dated 11-8-65
 AS-1-67 dated 1-11-68


Supplemental Specifications: 803 dated 1-1-69
 811 dated 1-1-69
 836 dated 6-17-69

DESIGN SPECIFICATIONS: This structure conforms to "Standard Specifications for Highway Bridges" adopted by the American Association of State Highway Officials, 1965, including the Ohio "Supplement" to these Specifications.

DESIGN LOADING: AASHTO HS 20-44

CONCRETE: unit stress 1200 p.s.i. for superstructure
 unit stress 1333 p.s.i. for substructure

STRUCTURAL STEEL: ASTM A-36 - basic unit stress = 20,000 p.s.i.

WELDS on Non-stress carrying members are shown thus: 

JOINT SEALER: Item 828 joint sealer including bond breaker, shown in Section A-A of Std. Dwg. SD-1-65, sheet No. 1, shall be omitted.

REINFORCING STEEL: ASTM A615, A616, A617, Deformed, Intermediate or Hard Grade. Unit stress 20,000 p.s.i.
 Spiral reinforcement shall be plain bars, A306 or A499.

PILES shall be driven to a minimum bearing capacity of 30 tons per pile for the abutments and 40 tons per pile for the piers.

MACHINE FINISH: The concrete deck shall be finished by the use of a finishing machine.

UTILITY LINES: All expenses involved in relocating the affected utility lines shall be borne by the owners. The Contractors and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either is held to a minimum.

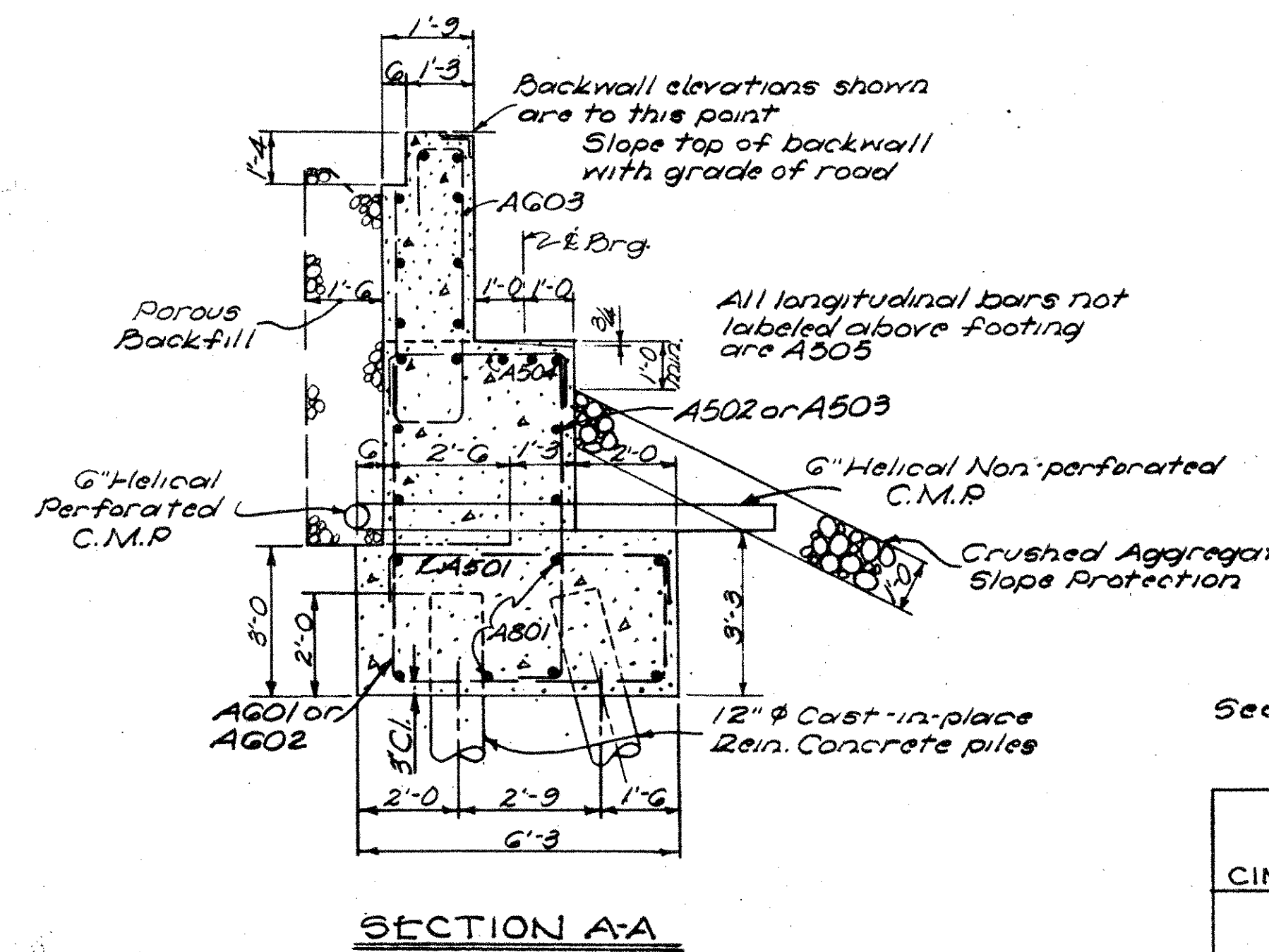
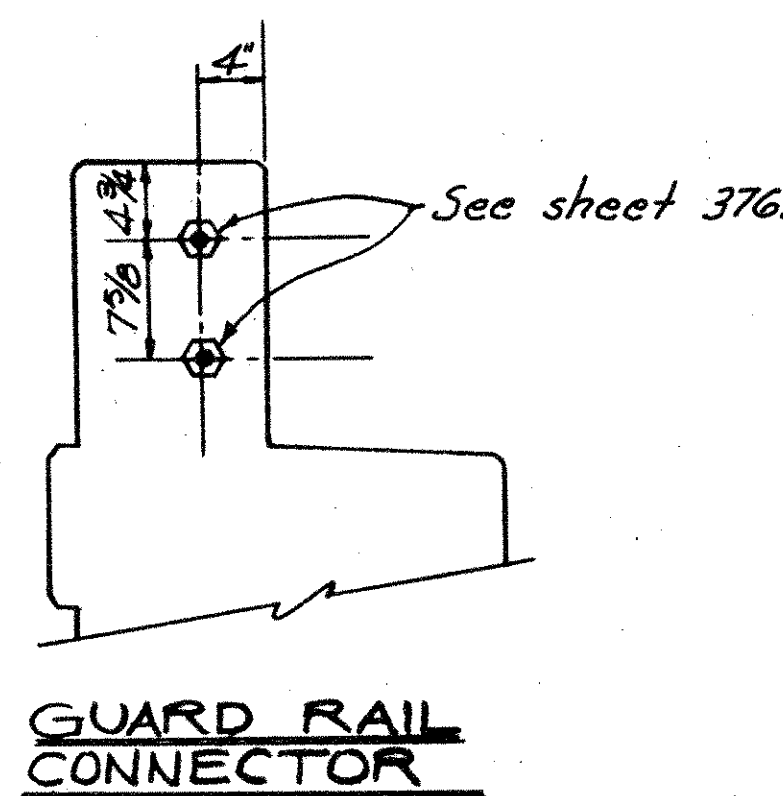
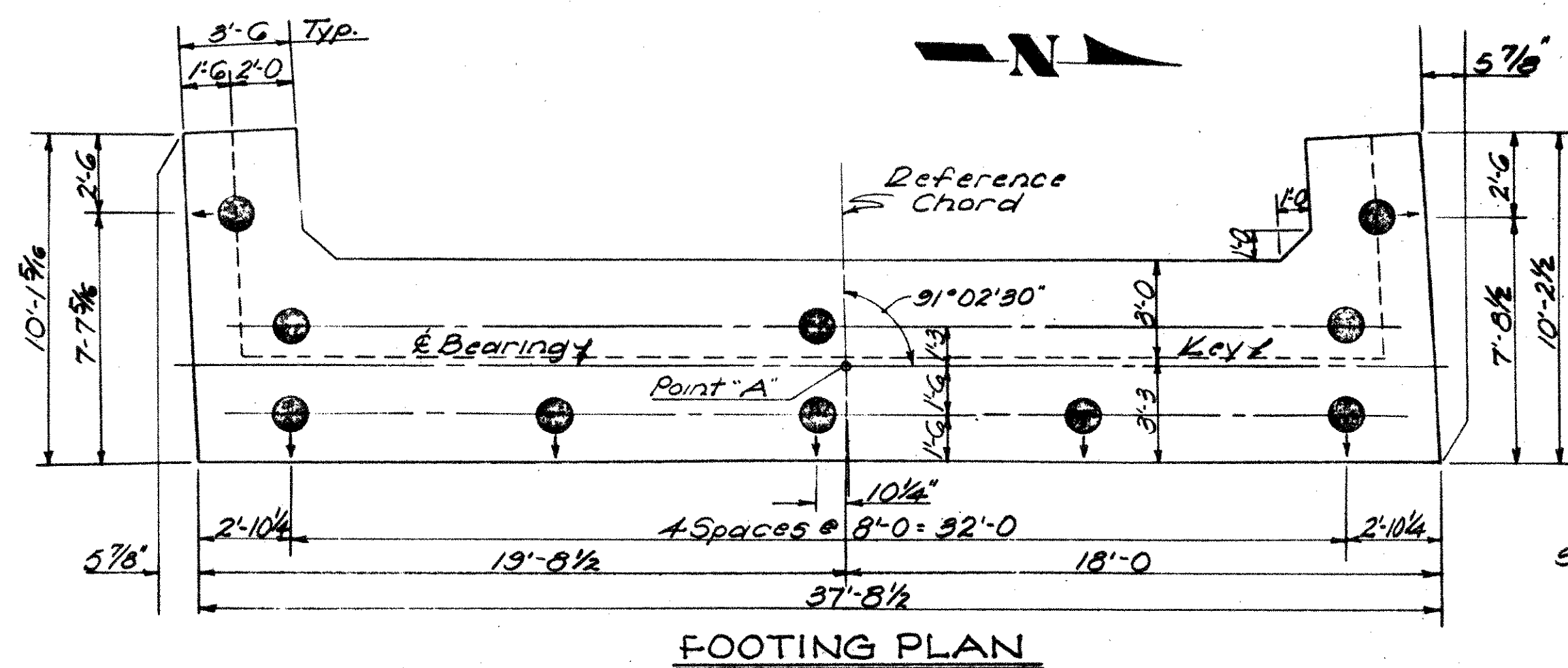
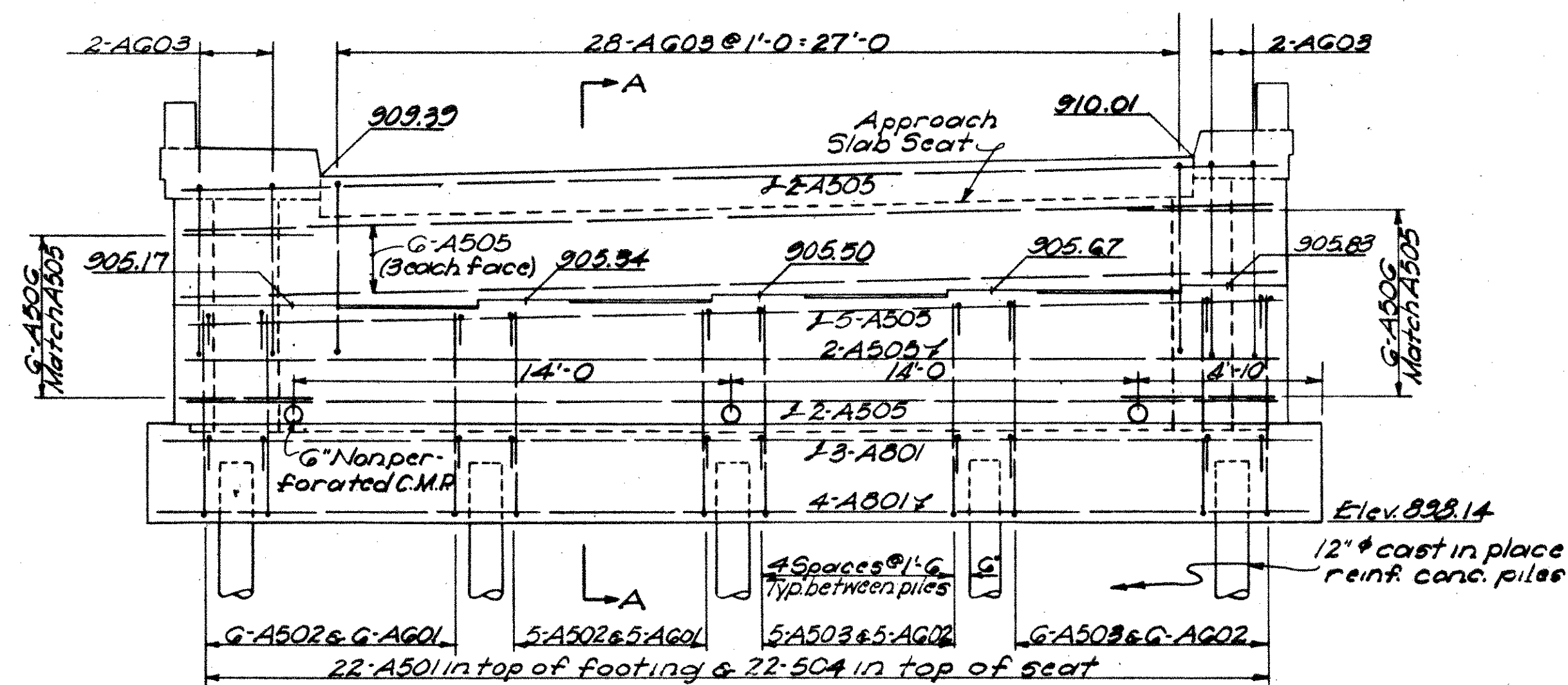
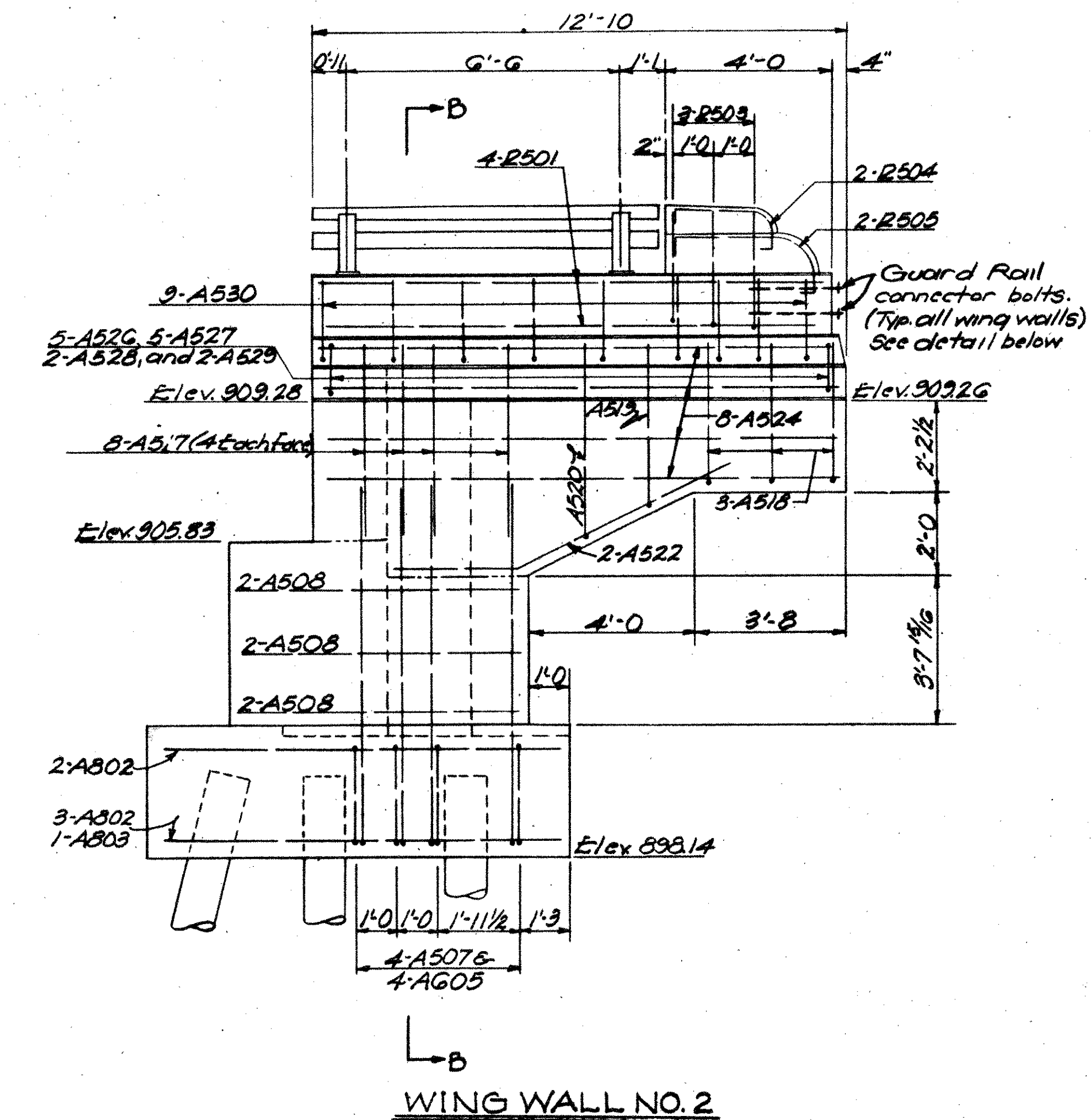
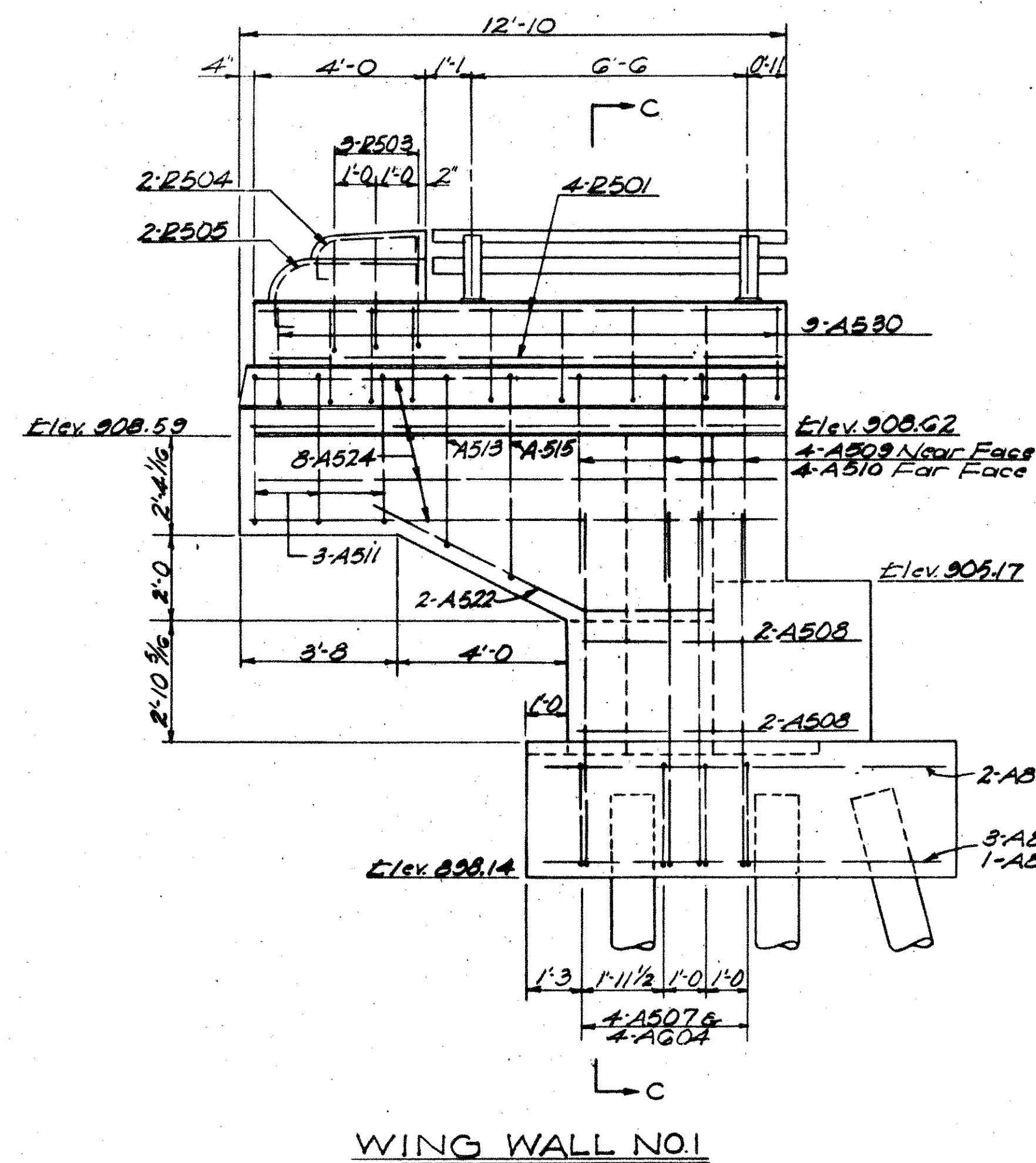
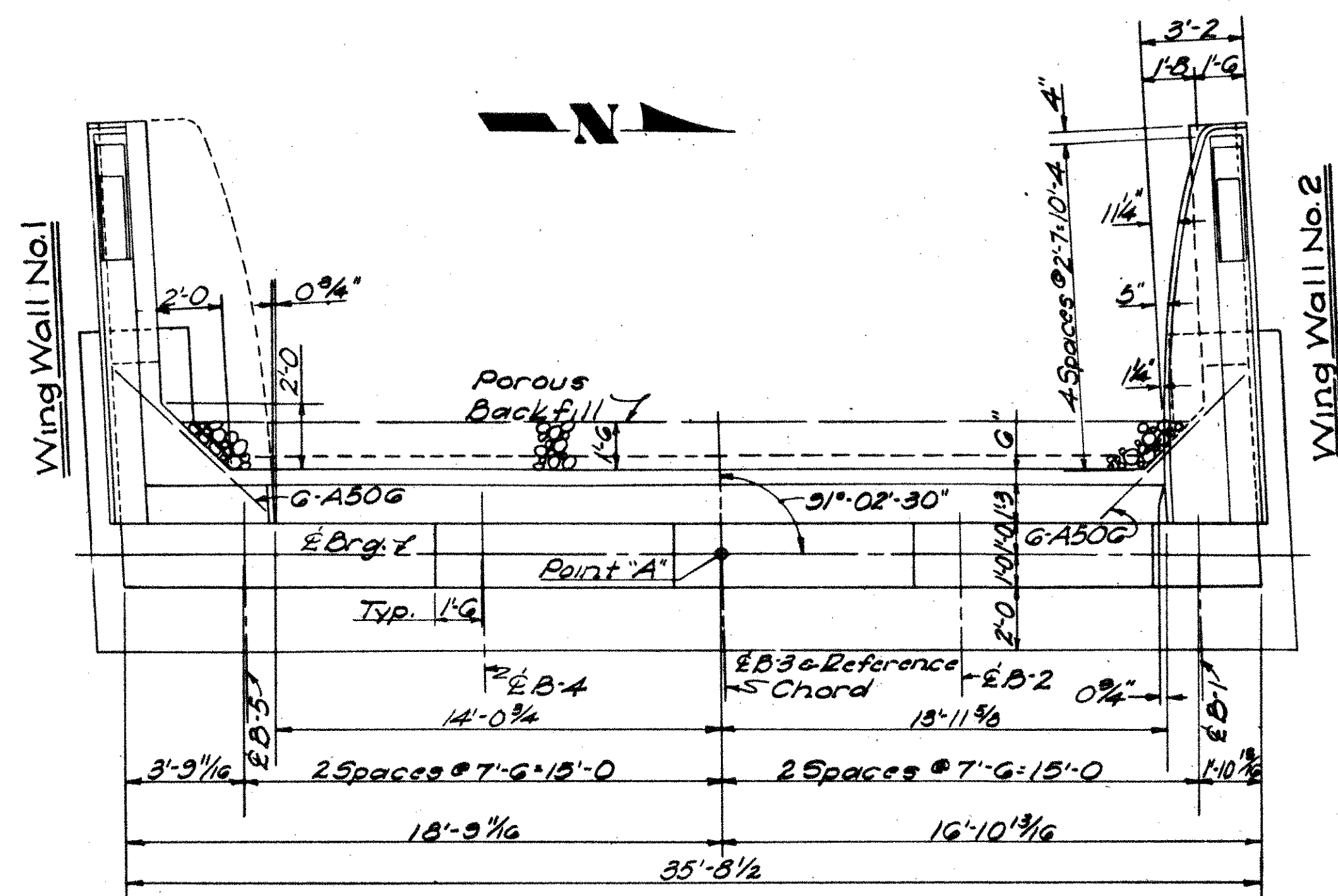
EXCAVATION QUANTITY includes the removal of fill materials required for construction of the abutments.

WELDED ATTACHMENTS: No attachments shall be made by field welding to the top flanges or flange plates of continuous beams or plate girders within a distance of 0.10 of the span length on either side of the interior supports. Welding for attachments to the top flanges at other parts of the spans shall be kept at least 2" from edge of flange.

PROCEDURE: All roadway embankments shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade for a distance of 200 feet in back of the abutments after which excavation shall be made and piles driven for both abutments and piers 1 and 4.

SCUPPERS including support angles, shall be galvanized in accordance with 711.02

SHAW, LENZ & ASSOCIATES 3/13			
ENGINEERS			
CINCINNATI			OHIO
QUANTITIES & GENERAL NOTES			
BRIDGE NO. MOT-75-1979			
STOP EIGHT OVER 1-75			
MONTGOMERY CO.		STA. 185+82.49	
DESIGNED	DRAWN	TRACED	CHECKED
W.B.S.	W.B.S.	W.T.R.	R.J.L.
			12-29-67

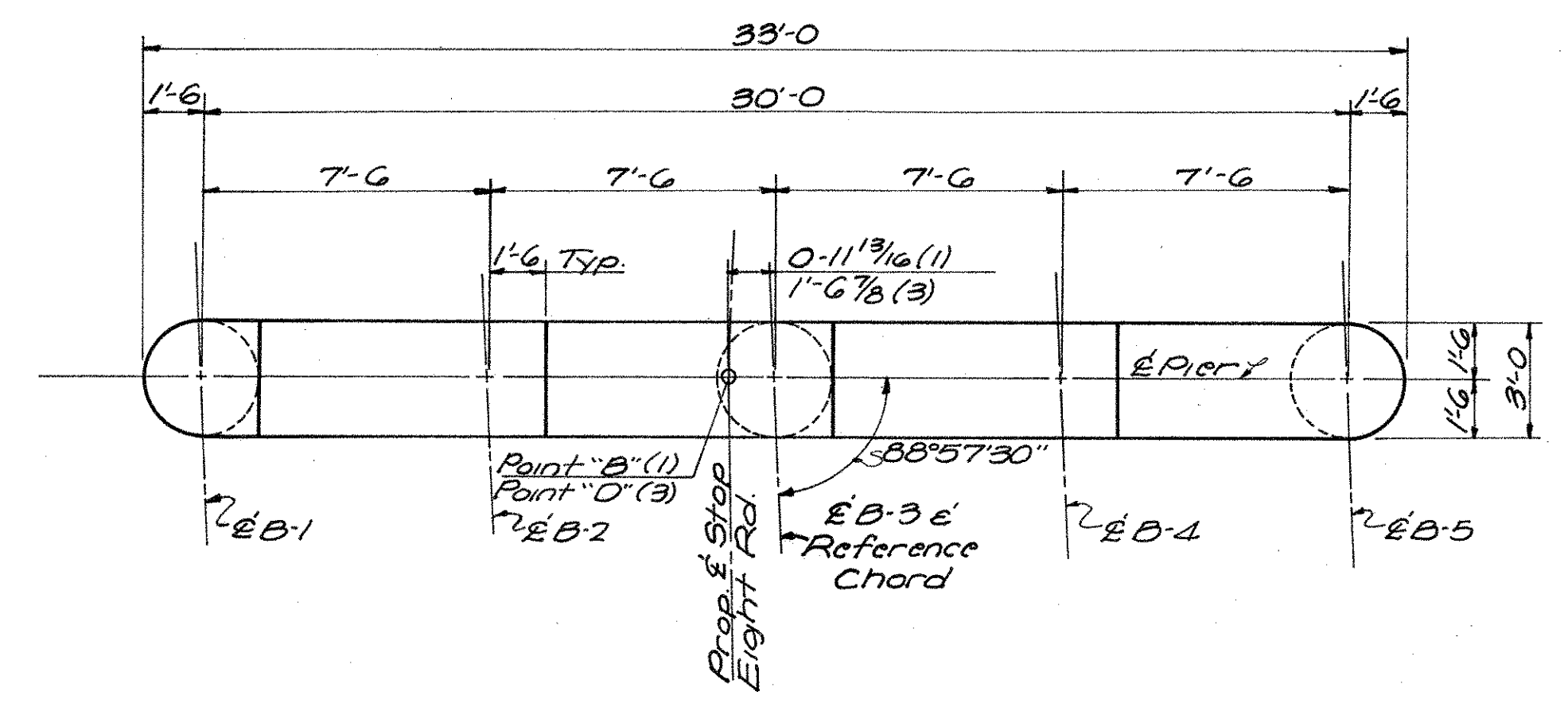


Note: For Sections B-B and C-C and additional notes see next sheet.

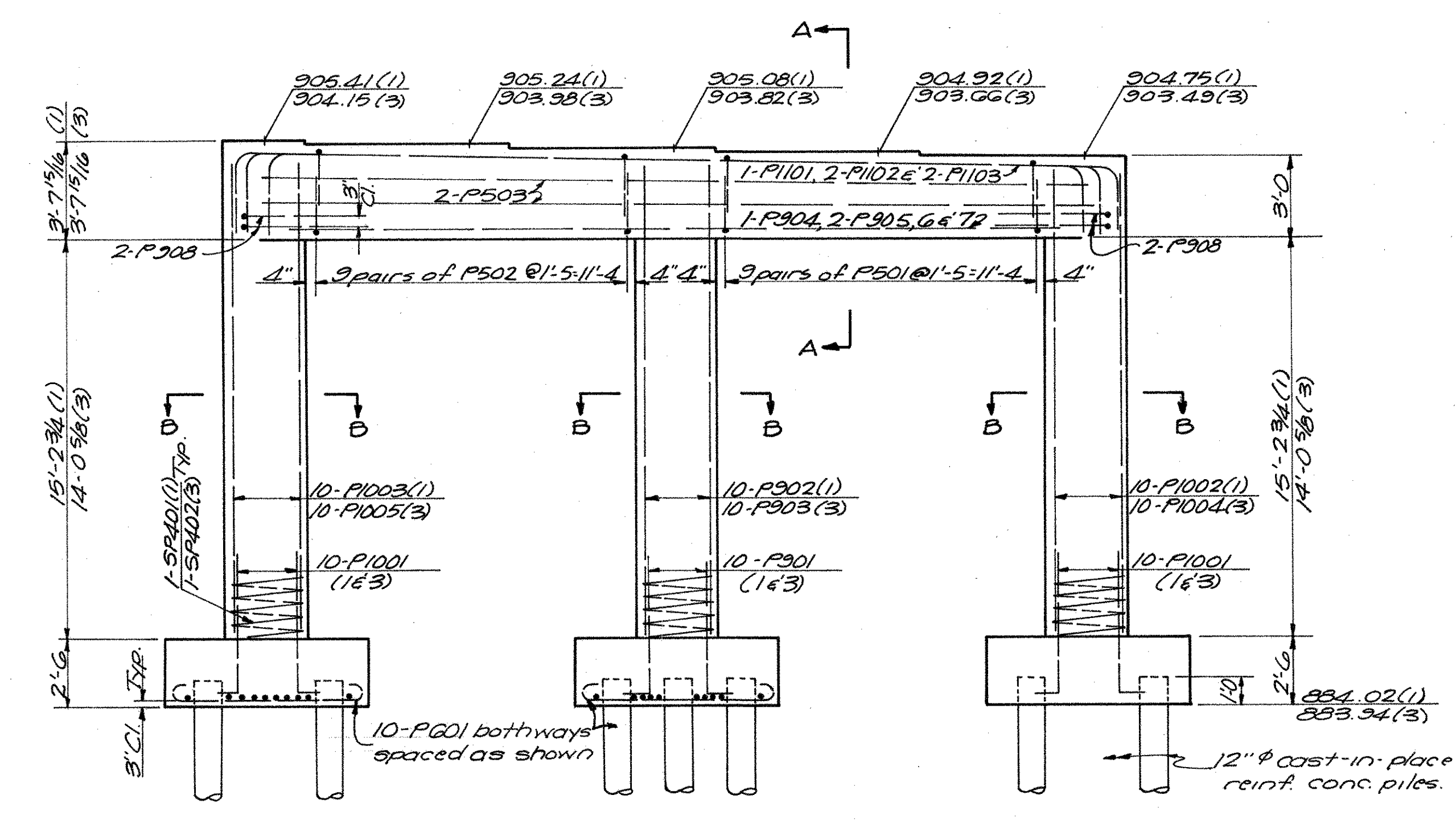
See Legend on next sheet.

See notes sheet 5

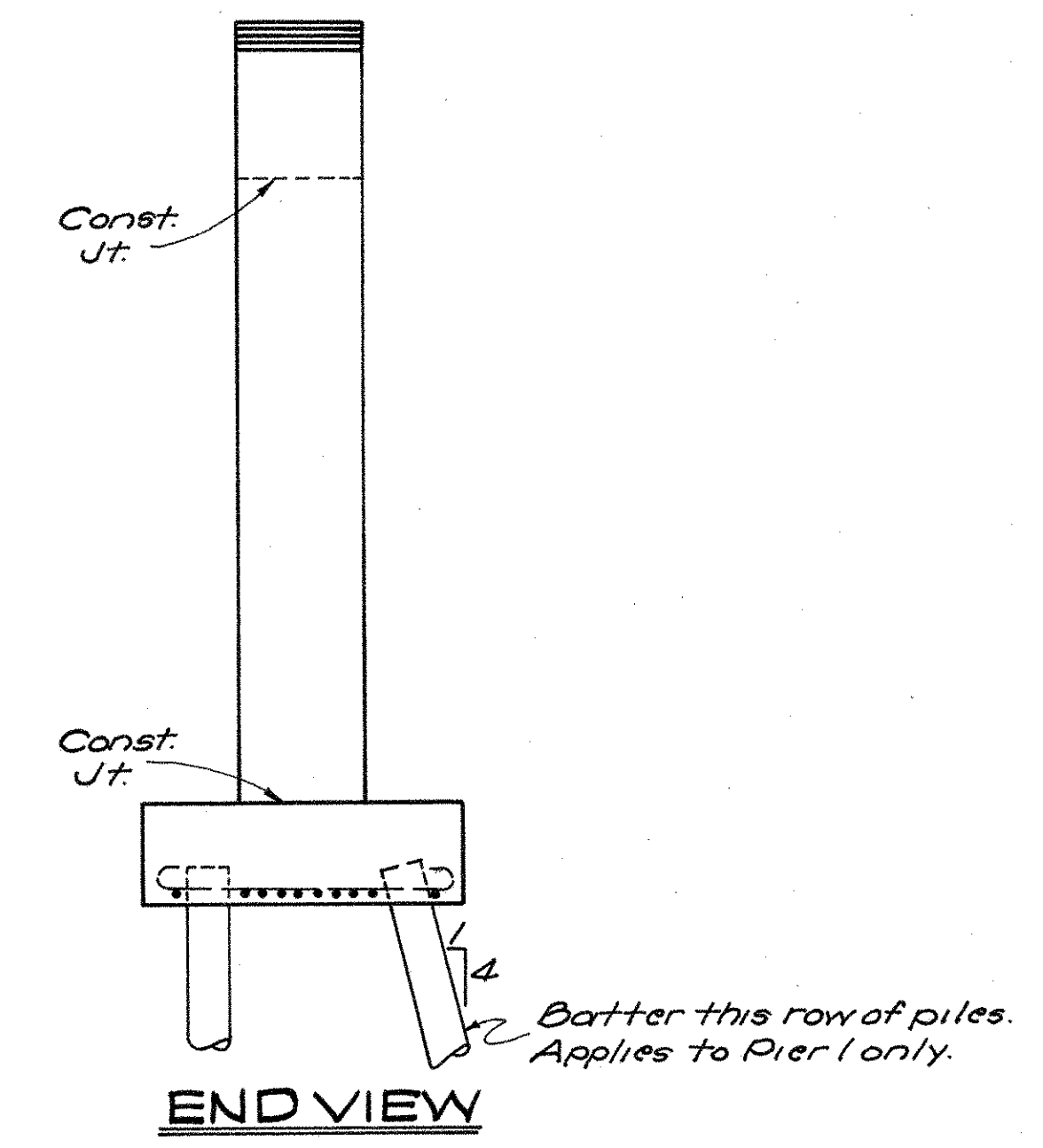
SHAW, LENZ & ASSOCIATES		4/13
ENGINEERS		
CINCINNATI		OHIO
ABUTMENT NO. 1		
BRIDGE NO. MOT-75-1975		
STOP EIGHT OVER 1-75		
MONTGOMERY CO.	STA. 165+82.40	
DESIGNED	DRAWN	TRACED
W.B.S.	W.B.S.	W.T.P.
		12-29-67



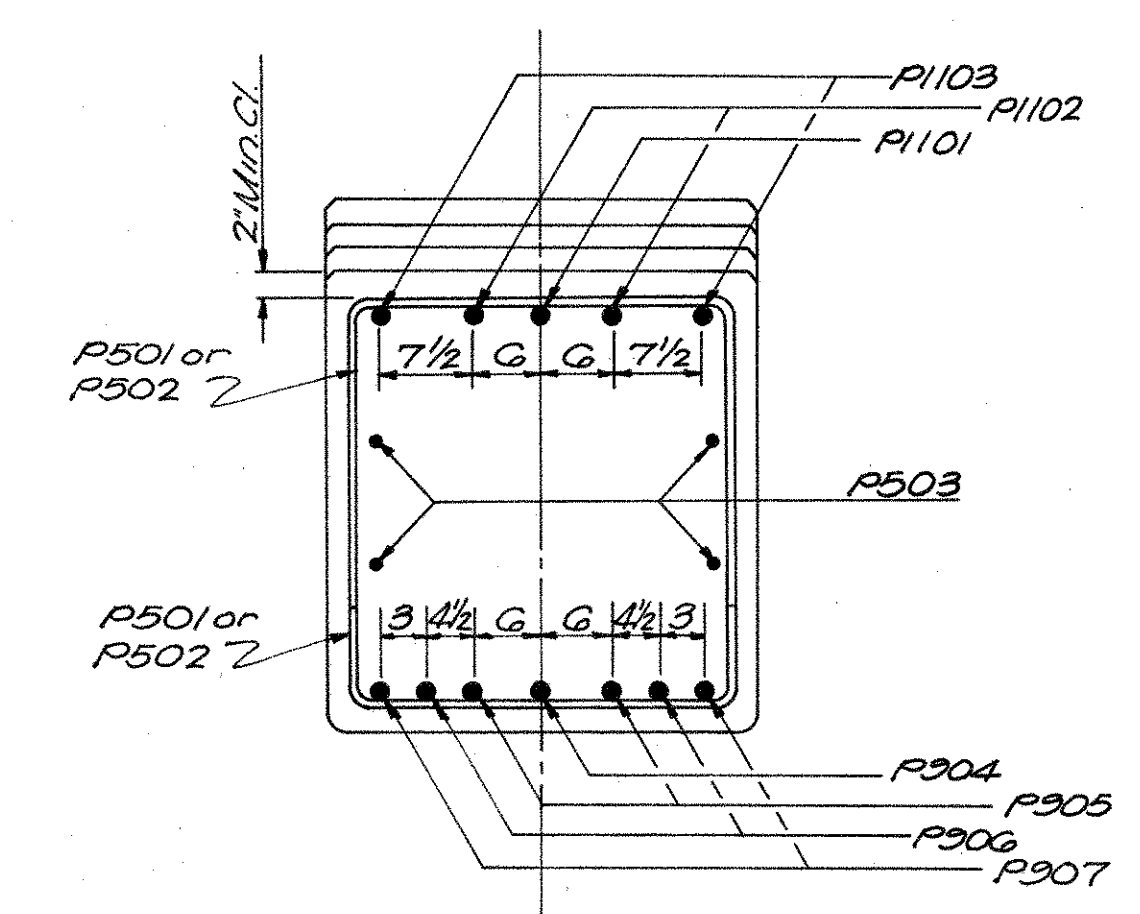
PLAN



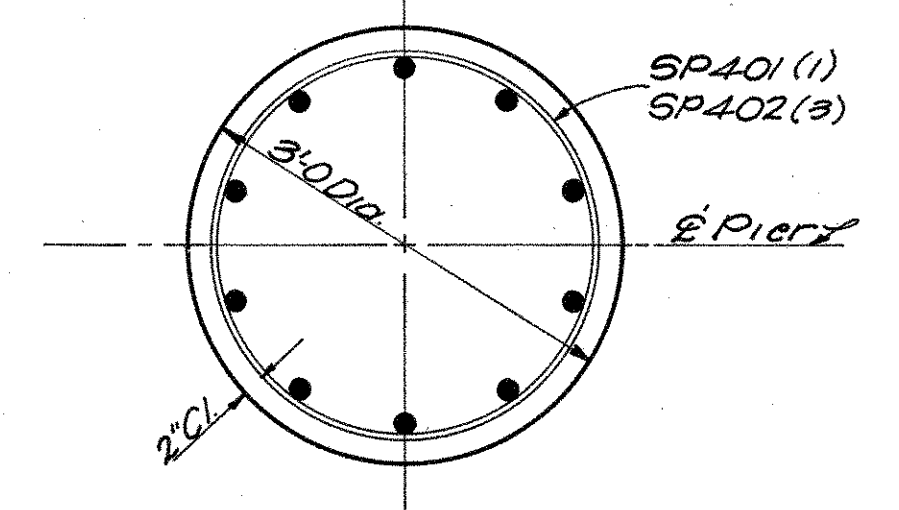
ELEVATION



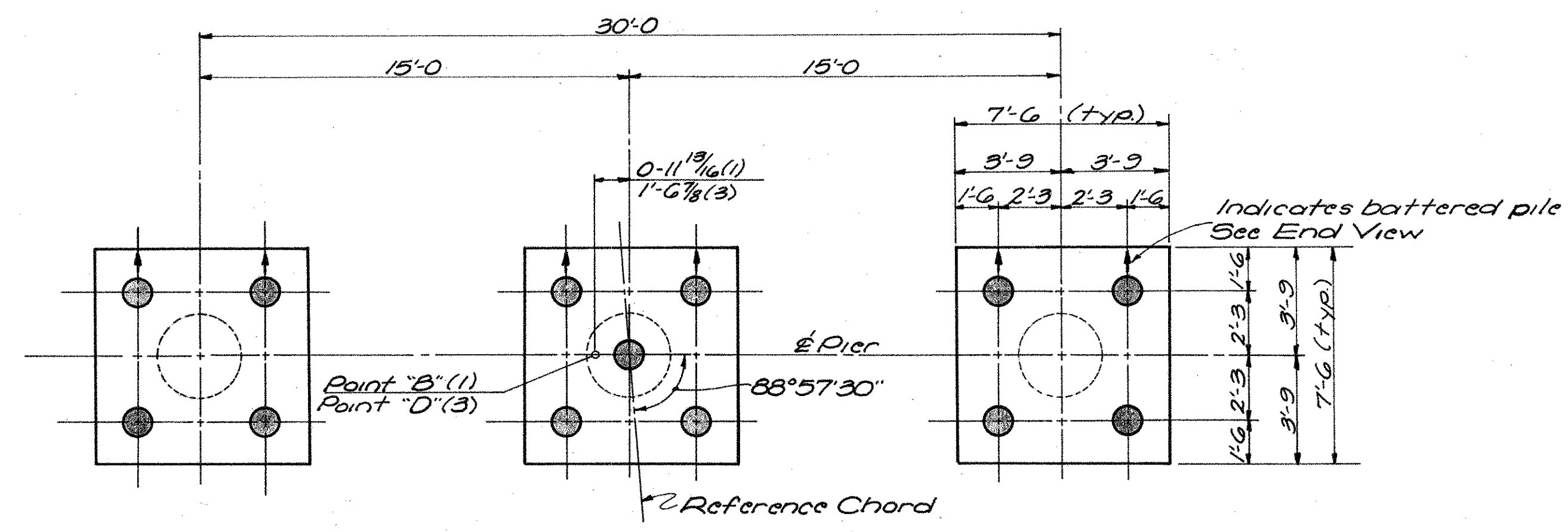
END VIEW



SECTION A-A



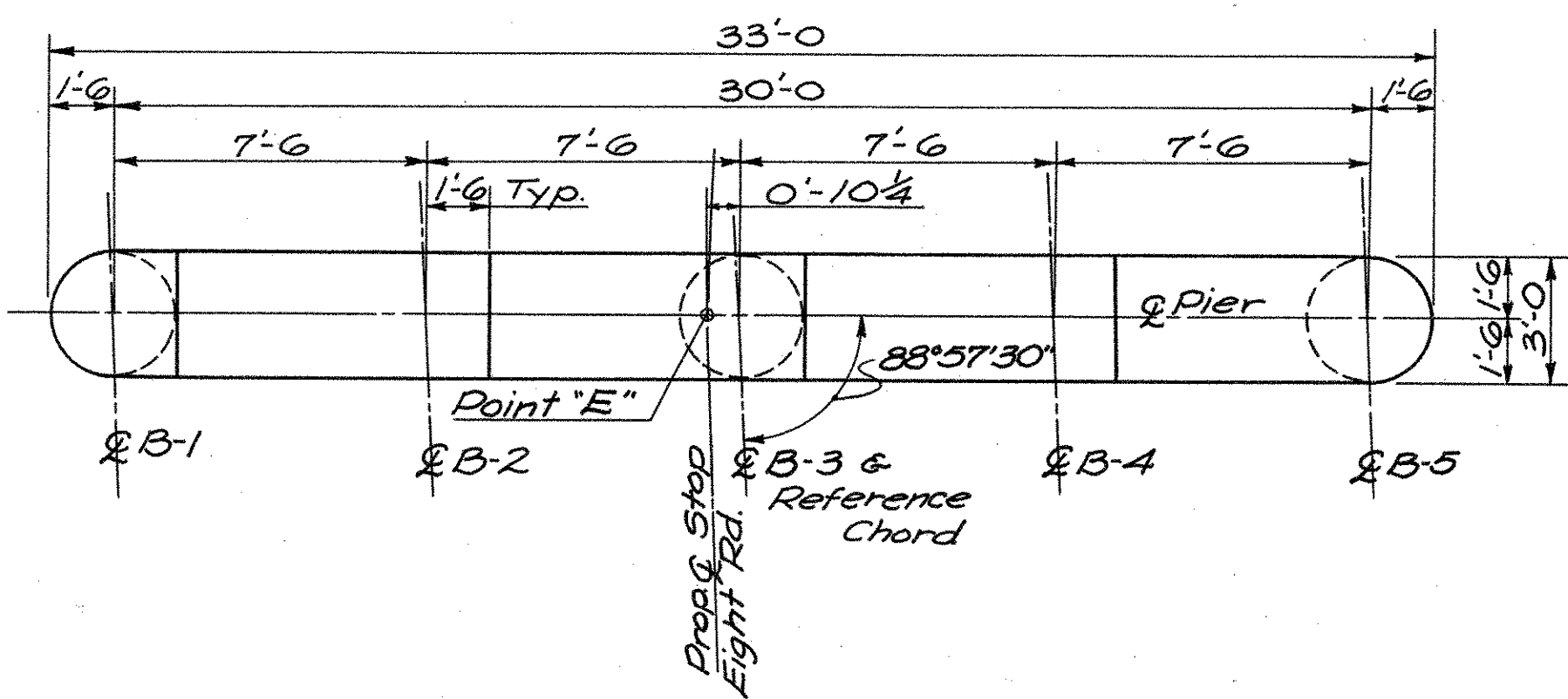
SECTION B-B



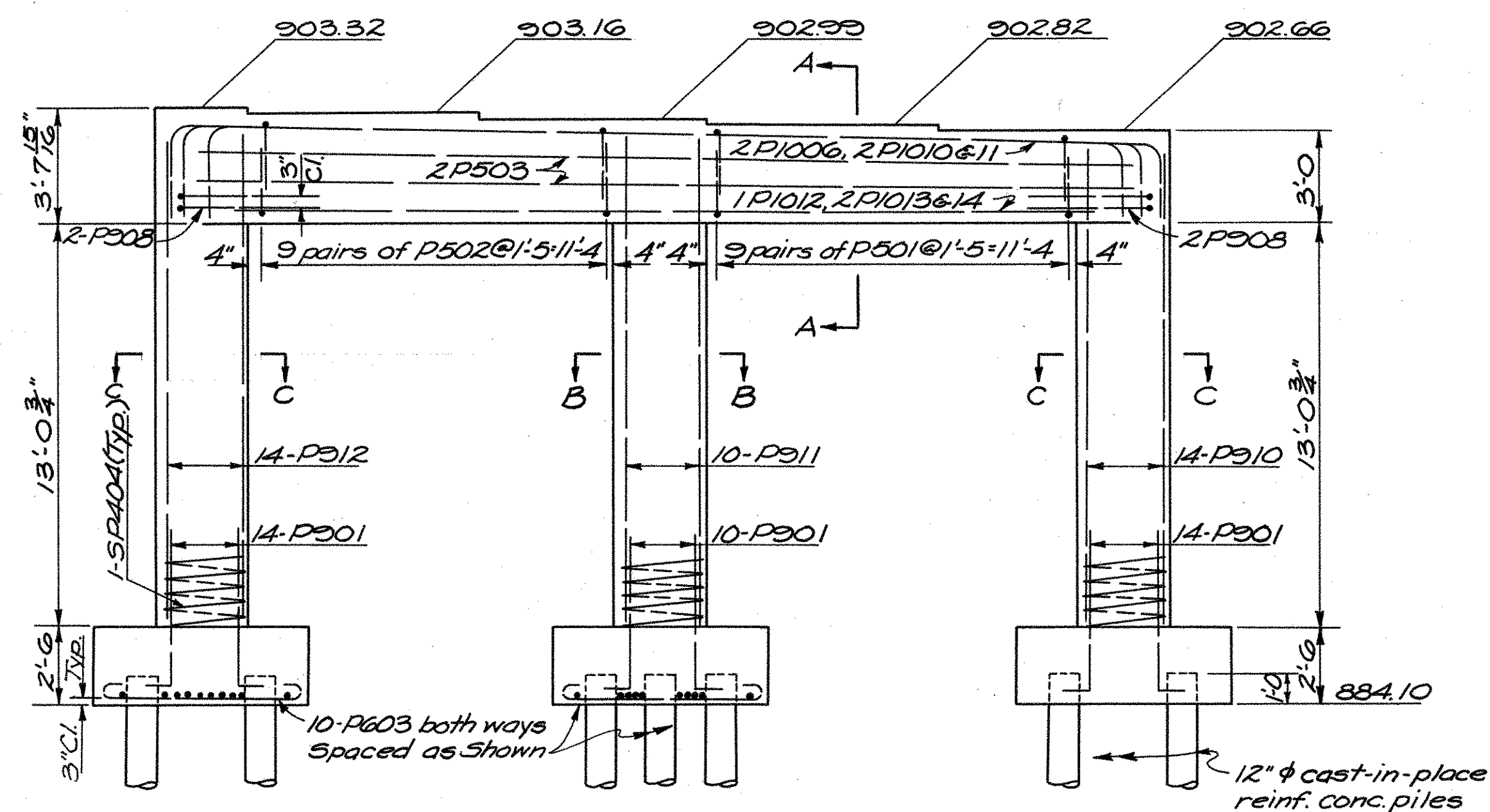
FOOTING PLAN

Notes: For location of points "B" and "D" see Geometric Layout on Sht.
 (1) Refers to Pier 1
 (3) Refers to Pier 3
 Column steel must be placed as shown to avoid interference with cap steel.

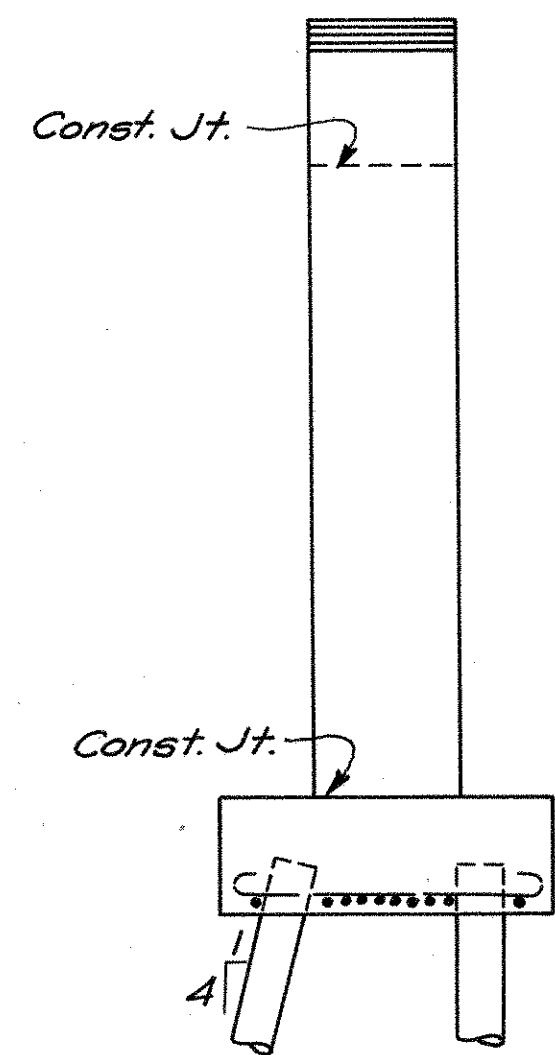
SHAW, LENZ & ASSOCIATES ENGINEERS					6/13
CINCINNATI					OHIO
PIERS 1 & 3					
BRIDGE NO. MOT. 75-1975 STOP EIGHT OVER I-75					
MONTGOMERY CO. STA. 185+82.43					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	W.B.S.	W.T.R.	R.J.L.		12-29-67



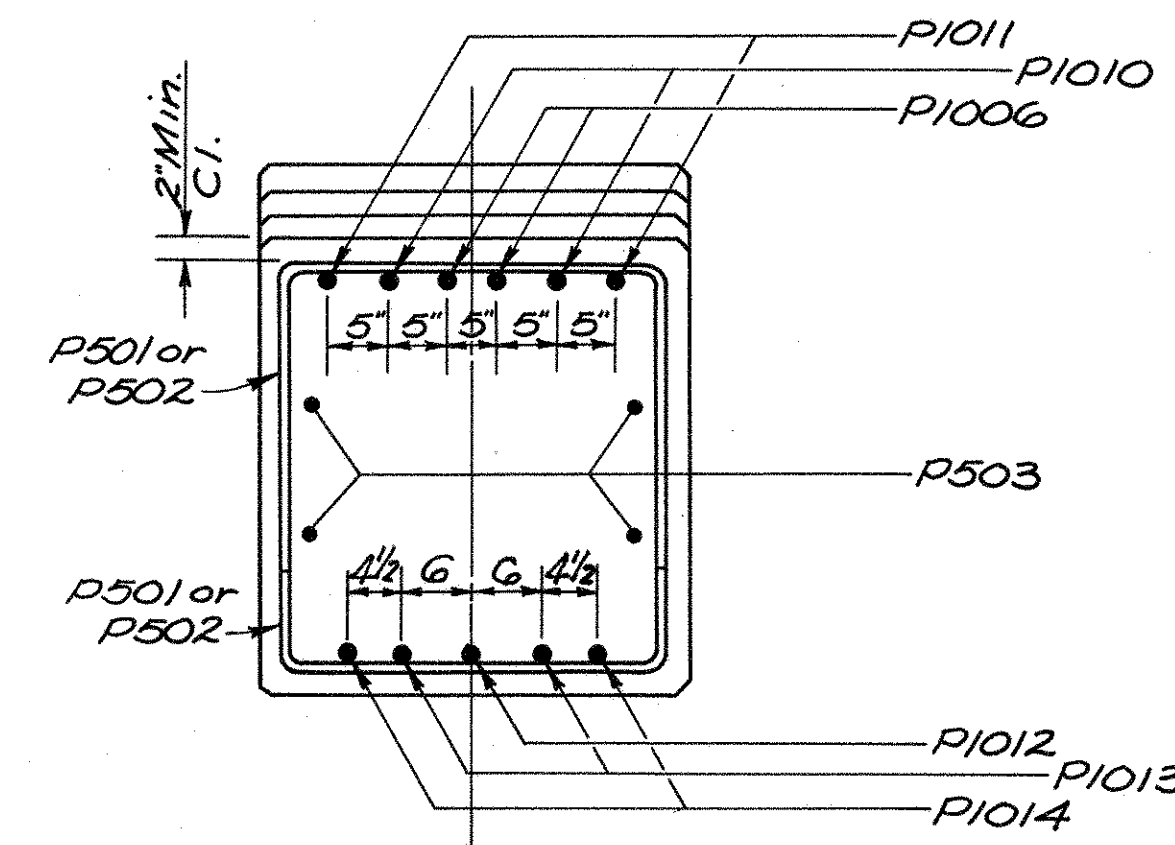
PLAN



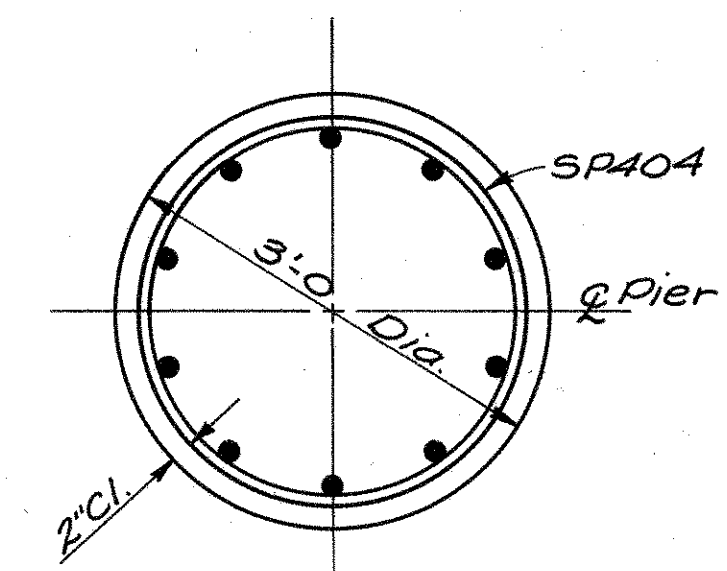
ELEVATION



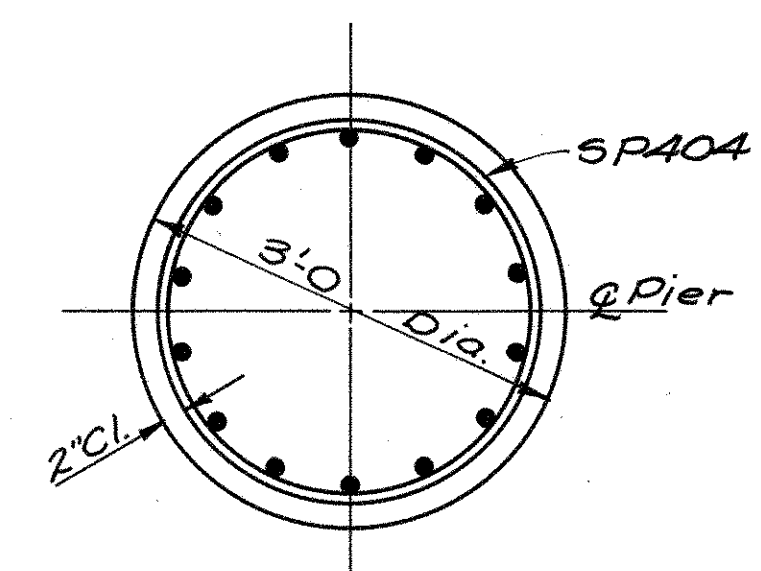
END VIEW



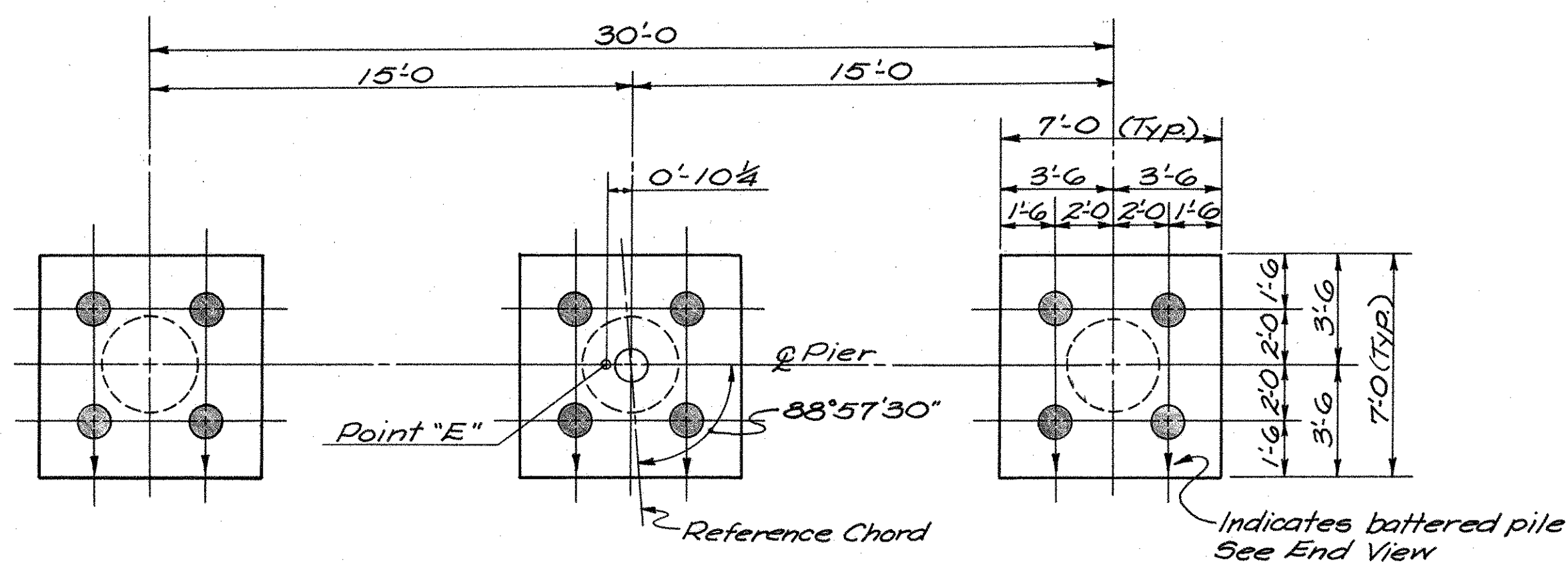
SECTION A-A



SECTION B-B



SECTION C-C

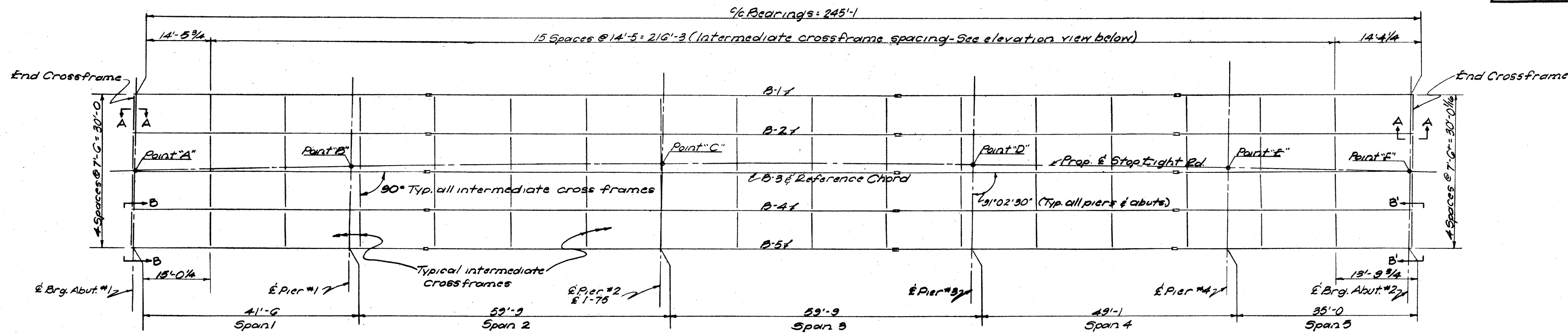


FOOTING PLAN

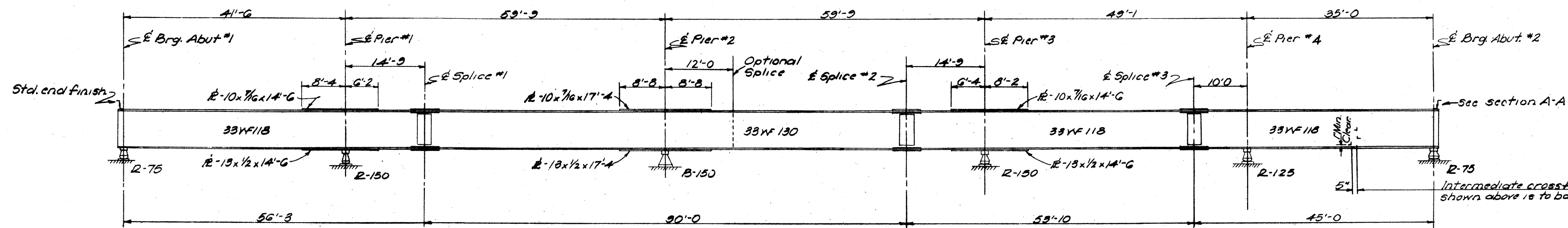
SHAW, LENZ & ASSOCIATES ^{8/13}
 ENGINEERS
 CINCINNATI OHIO

PIER 4
 BRIDGE NO. MOT-75-1979
 STOP EIGHT OVER I-75

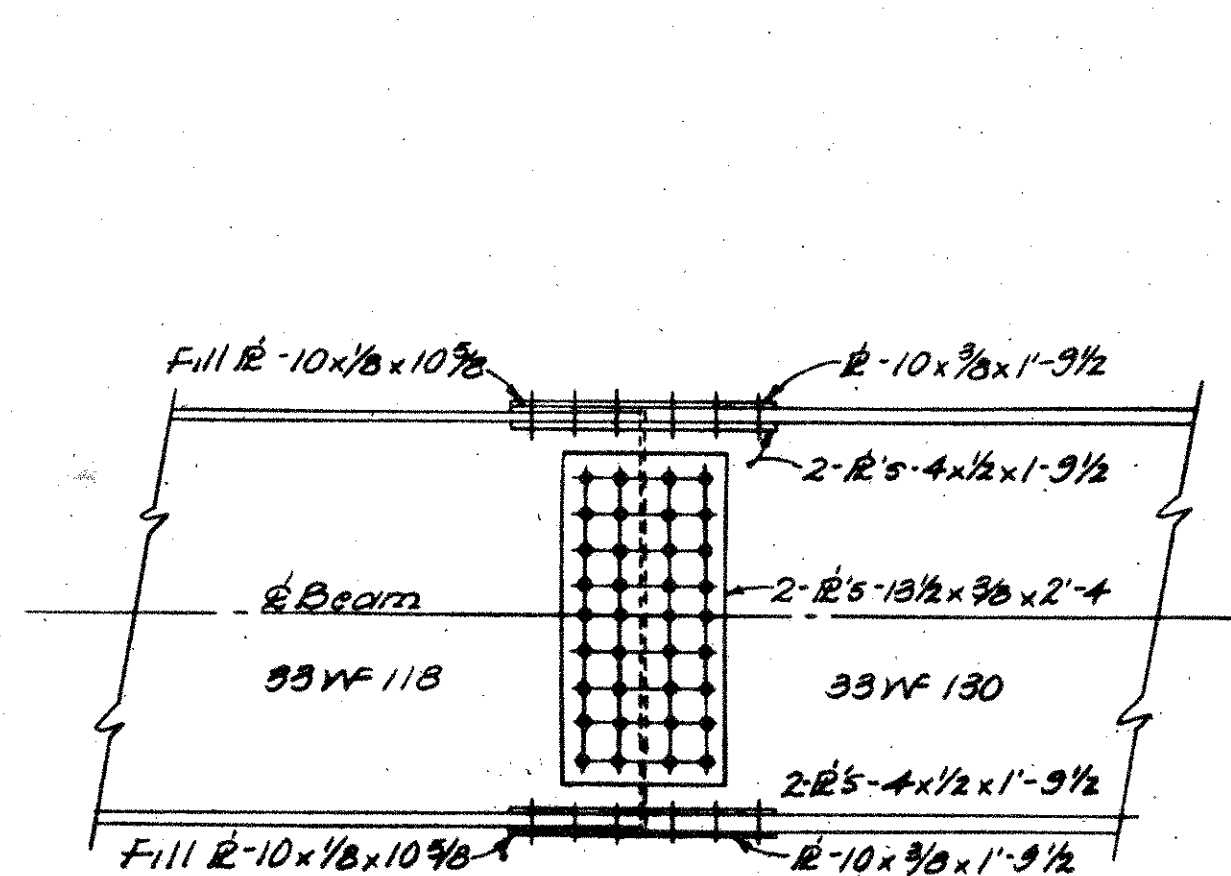
MONTGOMERY CO. STA. 185+82.49
 DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
 W.B.S. W.B.S. W.T.R. R.J.L. 12-29-67



FRAMING PLAN

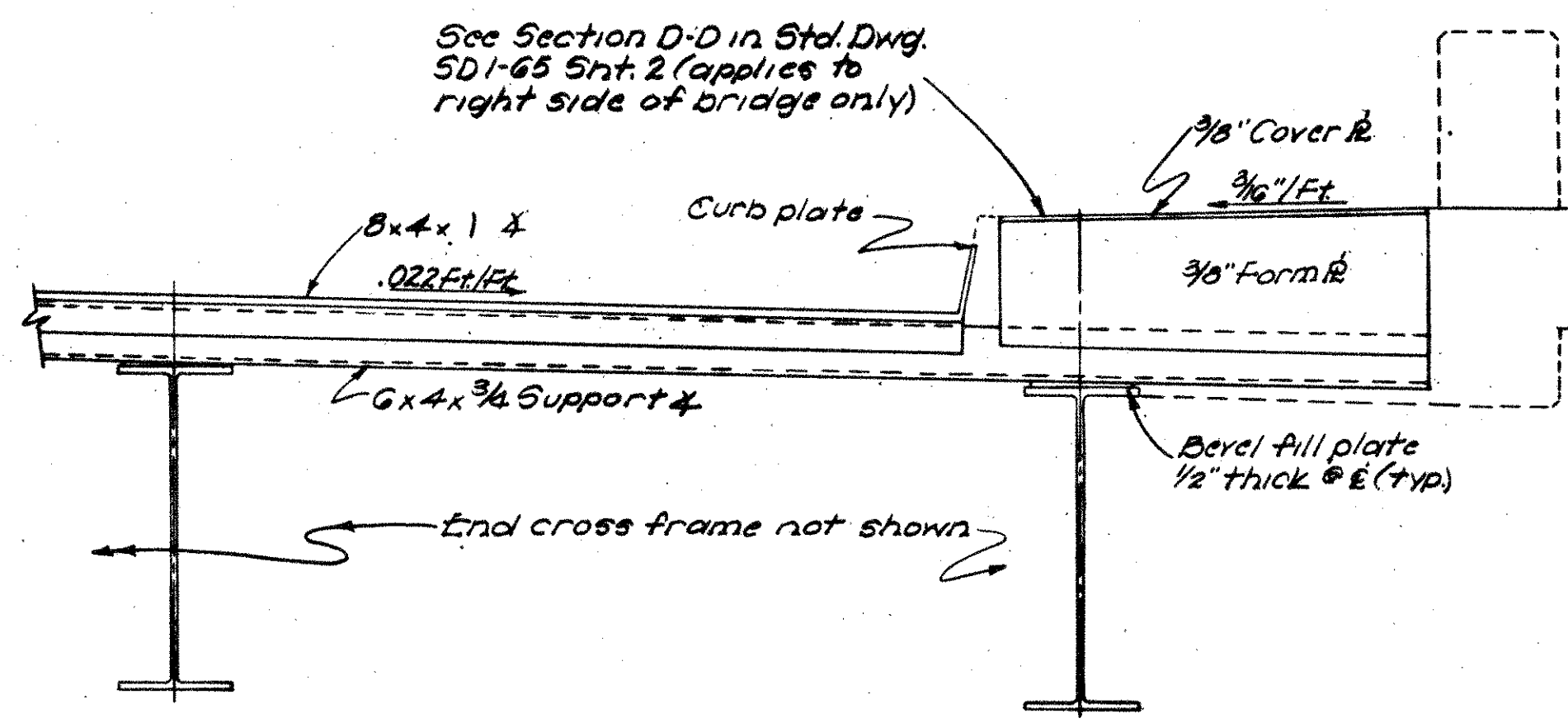


TYPICAL BEAM ELEVATION

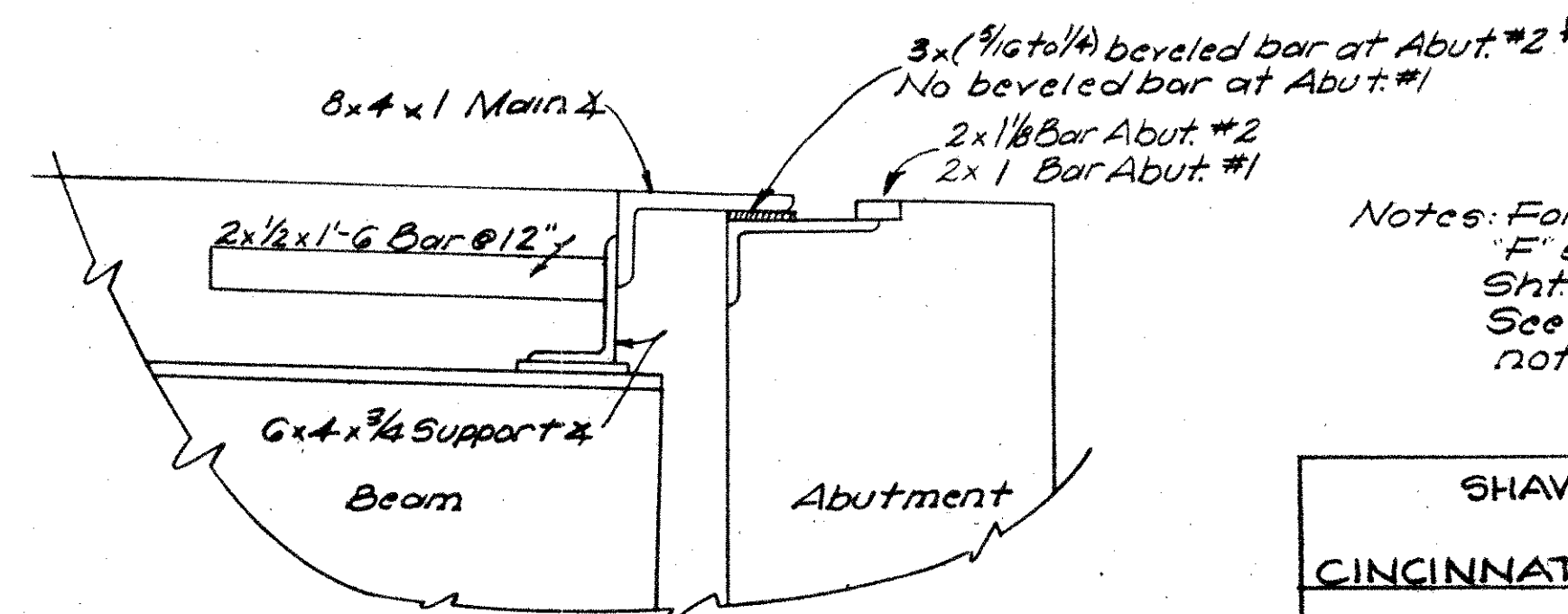


SPlice DETAIL

For details and dimensions not shown see Std. Dwg. SD1-65 Sht. 3. Flange fill plates used @ splices #1 & #2 only. Optional splice shall conform to std. splice for 33WF130 beam. Material for optional splice is not a pay item.



SECTION B-B
(Section B'-B' Similar)



SECTION A-A

* Use same size beveled bar at sidewalk with a 1/2 end bar

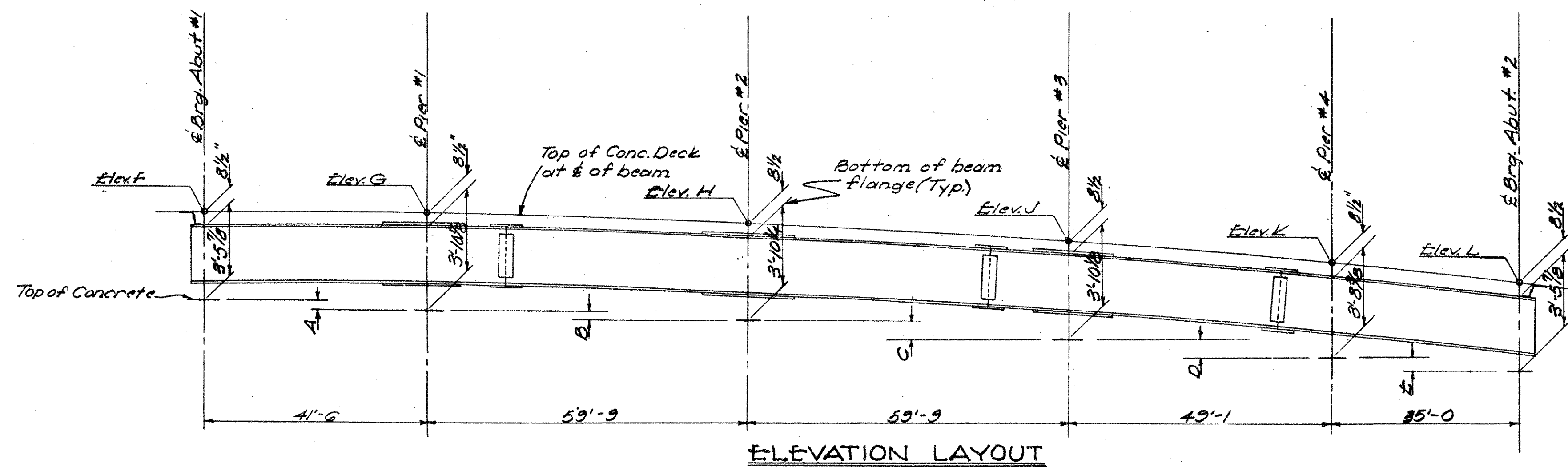
Notes: For location of Points A" thru F" see Geometric Layout on Sht. 2. See next sheet for additional notes and details.

SHAW, LENZ & ASSOCIATES g/13
ENGINEERS
CINCINNATI OHIO

FRAMING PLAN

BRIDGE NO. MOT-75-1979
STOP EIGHT OVER I-75

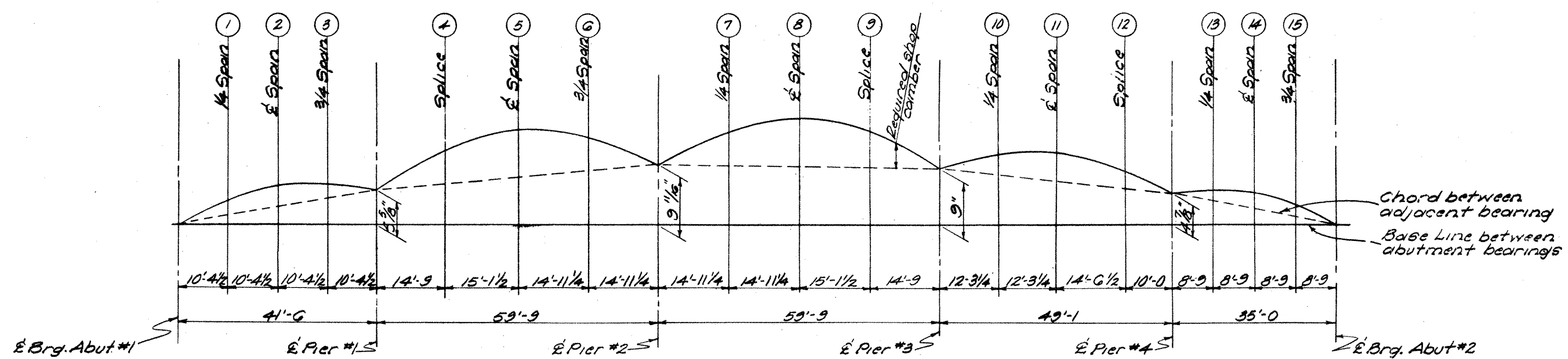
MONTGOMERY COUNTY STA. 185+82.49
DESIGNED DRAWN TRACED CHECKED REVIEWED/REVISED
W.B.S. W.B.S. W.T.R. R.J.L.
12-29-67



Beam	Dim. A	Dim. B	Dim. C	Dim. D	Dim. E	Elev. F	Elev. G	Elev. H	Elev. J	Elev. K	Elev. L
B-1	5 1/8"	5 1/8"	9 13/16"	9 15/16"	7 7/8"	909.03*	909.96*	909.53	908.70	907.73*	906.87*
B-2	5 1/16"	5 1/16"	9 13/16"	9 13/16"	7 13/16"	909.87	909.80	909.36	908.54	907.56	906.70
B-3	5 1/16"	5 1/16"	9 13/16"	9 13/16"	7 13/16"	909.70	909.63	909.20	908.37	907.40	906.54
B-4	5 1/16"	5 1/16"	9 13/16"	10"	7 13/16"	909.53	909.47	909.04	908.21	907.23	906.37
B-5	5"	5 5/16"	9 13/16"	10"	7 7/8"	909.37*	909.30*	908.87*	908.04*	907.07*	906.20*

* These elevations are on top of conc. deck beyond curbline.

- Notes: 1. For scupper details see Std. Drwg. 5D-1-65, sheet 2.
 2. For scupper locations see Sht. 11.
 3. For end crossframes, roadway end dam, sidewalk end dam, and curb plates, see Std. Drwg. 5D-1-65 Sheets 1 & 2.
 4. For details of Rockers and Bolsters, see Std. Drwg. 2B-1-55.
 5. For intermediate crossframes see next sheet.
 6. For shop welding of moment plates, see Std. Drwg. 5D-1-65 Sheet 2.



Beam	Location	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
Exterior Lt B-1	Deflection due to D.L. beam	0	0	0	+1/32	+1/32	+1/32	+1/32	+1/32	+1/32	0	+1/32	0	0	0	0
	Deflection due to remaining D.L.	+3/32	+3/32	+1/32	+7/32	+1/32	+3/16	+3/16	+1/32	+7/32	+1/32	+1/8	+1/32	+1/32	+1/16	+1/16
	Vertical curve correction	+1/32	+3/16	+7/32	+13/32	+3/8	+13/32	+13/32	+3/8	+13/32	+3/16	+13/32	+9/32	+9/32	+7/32	+9/32
	Superelevation correction	0	0	0	-1/32	-1/32	-1/32	-1/32	-1/32	-1/32	0	-1/32	0	0	0	0
Required shop camber		+3/16	+7/16	+1/4	+1/16	+1	+1/16	+1/16	+1	+1/16	+3/8	+3/16	+3/16	+3/16	+3/16	+1/4
Interior B-2 B-3 B-4	Deflection due to D.L. beam	0	0	0	+1/32	+1/32	+1/32	+1/32	+1/32	+1/32	0	+1/32	0	0	0	0
	Deflection due to remaining D.L.	+3/32	+3/32	+1/32	+3/16	+3/16	+3/32	+3/32	+3/16	+7/32	+1/16	+3/8	+1/16	+1/32	+1/16	+1/16
	Vertical curve correction	+7/32	+3/16	+7/32	+13/32	+3/8	+13/32	+13/32	+3/8	+13/32	+3/16	+13/32	+9/32	+9/32	+7/32	+9/32
	Superelevation correction	0	0	0	-1/32	-1/32	-1/32	-1/32	-1/32	-1/32	0	-1/32	0	0	0	0
Required shop camber		+3/16	+7/16	+1/4	+1/16	+3/8	+3/8	+3/8	+3/8	+13/16	+3/8	+3/16	+3/16	+3/16	+3/16	+1/4
Exterior Rt B-5	Deflection due to D.L. beam	0	0	0	+1/32	+1/32	+1/32	+1/32	+1/32	+1/32	0	+1/32	0	0	0	0
	Deflection due to remaining D.L.	+3/32	+1/8	+1/32	+3/16	+3/16	+3/32	+3/32	+3/16	+7/32	+1/16	+3/8	+1/32	+1/32	+1/16	+1/16
	Vertical curve correction	+7/32	+3/16	+7/32	+13/32	+3/8	+13/32	+13/32	+3/8	+13/32	+3/16	+13/32	+9/32	+9/32	+7/32	+9/32
	Superelevation correction	0	0	0	-1/32	-1/32	-1/32	-1/32	-1/32	-1/32	0	-1/32	0	0	0	0
Required shop camber		+3/16	+7/16	+1/4	+1/16	+3/8	+3/8	+3/8	+13/16	+3/8	+3/16	+3/16	+3/16	+3/16	+3/16	+1/4

+ = Convex Camber
 - = Concave Camber

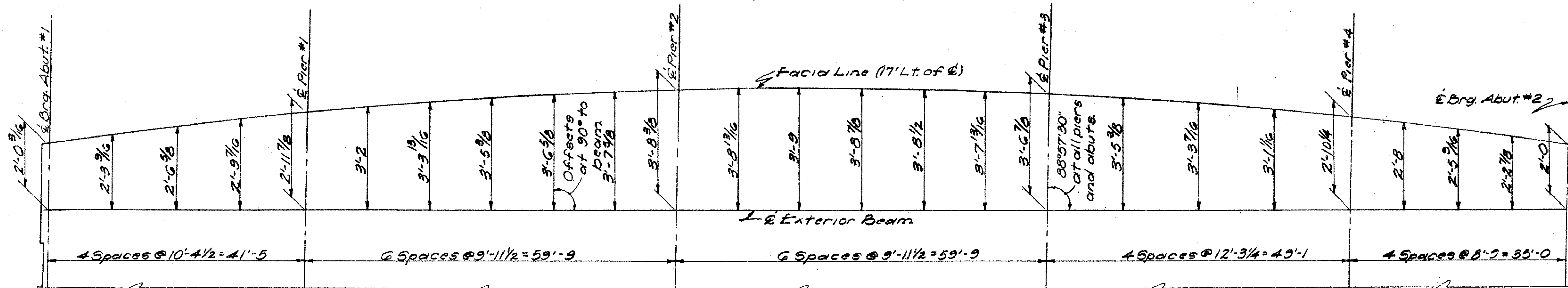
SHAW, LENZ & ASSOCIATES 10/13
 ENGINEERS
 CINCINNATI OHIO

STEEL DETAILS

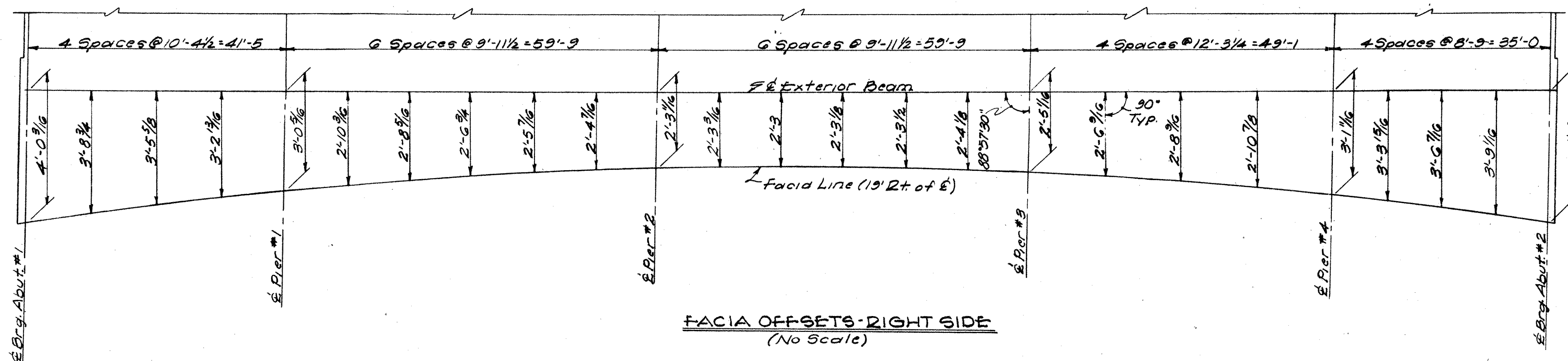
BRIDGE NO. MOT. 75-1979
 STOP EIGHT OVER 1-73

MONTGOMERY COUNTY STA. 185+82.49

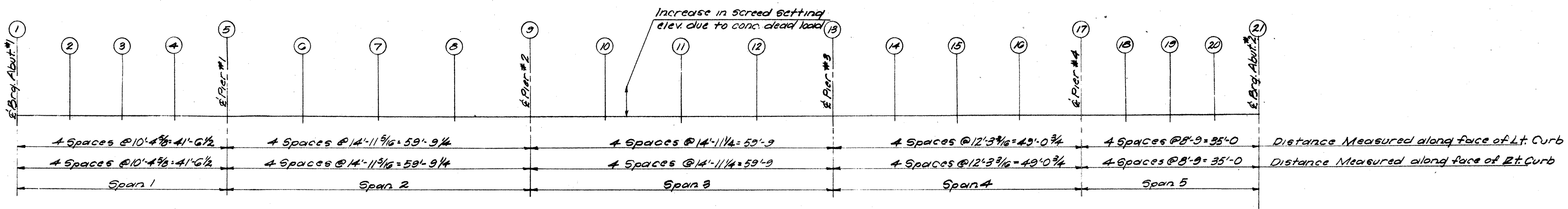
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	W.B.S.	W.T.R.	R.J.L.		
			12-23-67		



FACIA OFFSETS - LEFT SIDE
(No Scale)



FACIA OFFSETS - RIGHT SIDE
(No Scale)



SCREED ELEVATION DIAGRAM

Curb Line Location	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21
Left Curb																					
E. Station	19+08.75	19+19.10	19+29.45	19+39.80	19+50.15	19+60.50	19+70.85	19+81.20	20+09.72	20+24.61	20+39.50	20+54.39	20+69.28	20+84.17	20+99.06	21+13.95	21+28.84	21+43.73	21+58.62	21+73.51	21+88.40
E. Profile Grade	909.701	909.707	909.701	909.693	909.652	909.586	909.494	909.377	909.234	909.065	908.870	908.650	908.404	908.123	907.845	907.629	907.416	907.210	906.996	906.779	906.541
Deck elev. at face of curb	910.009	910.015	910.009	909.991	909.960	909.894	909.802	909.685	909.542	909.379	909.178	908.958	908.712	908.431	908.253	907.997	907.724	907.518	907.304	907.081	906.849
Conc. Dead Load Defl.	0.000	0.007	0.007	0.002	0.000	0.018	0.028	0.015	0.000	0.016	0.029	0.019	0.000	0.004	0.010	0.007	0.000	0.002	0.005	0.005	0.000
Screed Setting elev.	910.01	910.02	910.02	909.99	909.96	909.91	909.83	909.70	909.54	909.39	909.21	908.98	908.71	908.50	908.26	908.00	907.72	907.52	907.31	907.09	906.85
Right Curb																					
E. Station	19+07.44	19+17.86	19+28.28	19+38.69	19+49.11	19+59.52	19+70.09	19+80.65	20+09.07	20+24.06	20+39.05	20+54.03	20+69.02	20+84.01	20+99.00	21+14.00	21+29.00	21+44.00	21+59.00	21+74.00	21+89.00
E. Profile Grade	909.699	909.707	909.703	909.685	909.656	909.591	909.500	909.383	909.240	909.071	908.877	908.656	908.409	908.126	907.847	907.633	907.414	907.207	906.992	906.767	906.549
Deck elev. at face of curb	909.391	909.399	909.395	909.377	909.348	909.283	909.192	909.075	908.932	908.763	908.569	908.343	908.101	907.878	907.639	907.381	907.106	906.899	906.684	906.459	906.225
Conc. Dead Load Defl.	0.000	0.009	0.010	0.004	0.000	0.015	0.024	0.014	0.000	0.013	0.024	0.015	0.000	0.004	0.010	0.006	0.000	0.003	0.007	0.006	0.000
Screed Setting elev.	909.39	909.41	909.41	909.38	909.35	909.30	909.22	909.09	908.93	908.78	908.59	908.36	908.10	907.88	907.65	907.39	907.11	906.90	906.69	906.47	906.23

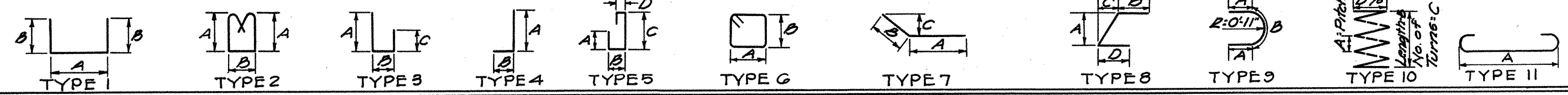
SHAW, LENZ & ASSOCIATES 12/13
ENGINEERS
CINCINNATI OHIO

SUPERSTRUCTURE DETAILS

BRIDGE NO. MOT. 75-1979
STOP EIGHT OVER I-75

MONTGOMERY COUNTY STA. 185+82.49
DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
W.B.S. W.B.S. W.T.R. E.J.L. 12-29-67

REINFORCING STEEL BAR SCHEDULE



MARK	NO.	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION	
ABUTMENTS										
A801	14	37'-4"	Str.					1396	Footings 1 & 2	
A802	20	9'-8"	Str.					516	Footings 1 & 2	
A803	4	9'-8"	7	5'-8"	4'-0"	1'-3"		103	Footings 1 & 2	
A601	22	14'-3"	3	6'-7"	5'-5"	2'-7"		471	Footings 1 & 2	
A602	22	14'-7"	3	6'-11"	5'-5"	2'-7"		452	Footings 1 & 2	
A603	64	12'-9"	5	4'-3"	1'-5"	5'-6"	0'-11"	1226	Backwalls 1 & 2	
A604	8	17'-6"	1	1'-2"	8'-4"			210	Footings 1 & 2	
A605	8	18'-10"	1	1'-2"	9'-0"			226	Footings 1 & 2	
A501	44	6'-11"	1	5'-5"	0'-10 1/2"			317	Footings 1 & 2	
A502	22	7'-4"	4	6'-7"	0'-10 1/2"			168	Footings 1 & 2	
A503	22	7'-8"	4	6'-11"	0'-10 1/2"			176	Footings 1 & 2	
A504	44	4'-11"	1	3'-5"	0'-10 1/2"			226	Seats 1 & 2	
A505	34	35'-4"	Str.					1253	Seat & Bkwall 1 & 2	
A506	24	6'-4"	Str.					159	Fillet 1 & 2	
A507	16	12'-0"	6	3'-2"	2'-7"			200	Footings 1 & 2	
A508	20	6'-9"	Str.					141	W.W. 1, 2, 3 & 4	
A509	8	7'-7"	3	4'-11"	0'-6"	2'-6"		63	W.W. 1 & 4	
A510	8	3'-3"	Str.					27	W.W. 1 & 4	
A511	5	9'-3"	5	2'-0"	1'-2"	3'-8"	0'-6"	43	W.W. 1 & 4	
A512	1	9'-5"	↑	2'-1"	↑	3'-9"	↑	10	W.W. 4	
A513	1	10'-5"	↑	2'-7"	↑	4'-3"	↑	11	W.W. 1	
A514	1	10'-9"	↑	2'-9"	↑	4'-5"	↑	11	W.W. 4	
A515	1	11'-11"	↓	3'-4"	↓	5'-0"	↓	12	W.W. 1	
A516	1	12'-3"	5	3'-6"	1'-2"	5'-2"	0'-6"	13	W.W. 4	
A517	16	4'-10"	Str.					31	W.W. 2 & 3	
A518	6	7'-10"	1	1'-2"	3'-6"			49	W.W. 2 & 3	
A519	2	8'-10"	1	1'-2"	4'-0"			18	W.W. 2 & 3	
A520	1	10'-4"	1	1'-2"	4'-9"			11	W.W. 2	
A521	1	10'-8"	1	1'-2"	4'-11"			11	W.W. 3	
A522	4	8'-9"	7	3'-0"	5'-9"	2'-7"		37	W.W. 1 & 2	
A523	4	8'-3"	7	3'-0"	5'-3"	2'-4"		34	W.W. 3 & 4	
A524	16	12'-4"	Str.					206	W.W. 1 & 2	
A525	16	11'-10"	Str.					197	W.W. 3 & 4	
A526	10	4'-9"	8	1'-3"	1'-10"	0'-2"	2'-0"	50	W.W. 2 & 3	
A527	10	3'-1"	1	1'-3"	1'-1"			32	W.W. 2 & 3	
A528	4	2'-9"	8	1'-3"	0'-10"	0'-2"	1'-0"	11	W.W. 2 & 3	
A529	4	2'-5"	1	1'-3"	0'-9"			10	W.W. 2 & 3	
A530	36	5'-3"	2	2'-0"	0'-8"			197	W.W. 1, 2, 3 & 4	
*R501	8	12'-2"	Str.						Railing	
*R502	8	11'-8"	Str.						Railing	
*R503	12									
*R504	8	See Std. Drwg. BR-1-65, sheet 2 for details								Railing
*R505	8								Railing	
PIERS										
P1101	2	38'-0"	8	32'-7"	2'-7 1/2"	0'-8"	3'-3 1/2"	404	Top Cap. 1 & 3	
P1102	4	37'-9"	8	32'-4"	2'-7 1/2"	0'-8"	3'-3 1/2"	802	Top Cap. 1 & 3	
P1103	4	35'-6"	8	30'-1"	2'-7 1/2"	0'-8"	3'-3 1/2"	754	Top Cap. 1 & 3	
P1104	1	31'-4"	Str.					166	Bot. Cap. 2	
P1105	2	30'-10"	Str.					328	Bot. Cap. 2	
P1106	2	30'-0"	Str.					319	Bot. Cap. 2	
P1001	60	7'-1"	4	5'-5"	1'-11"			1329	Footings 1, 2 & 3	
P1002	10	17'-8"	Str.					760	Column 1	
P1003	10	18'-4"	Str.					789	Column 1	
P1004	10	16'-6"	Str.					710	Column 3	
P1005	10	17'-2"	Str.					739	Column 3	
P1006	3	38'-0"	8	32'-6 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	491	Top Cap. 2 & 4	

MARK	NO.	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
PIERS Cont'd.									
P1007	2	37'-10"	8	32'-4 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	326	Top Cap. 2
P1008	2	37'-5"	8	31'-11 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	322	Top Cap. 2
P1009	2	35'-7"	8	30'-1 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	306	Top Cap. 2
P1010	2	37'-2"	8	31'-8 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	320	Top Cap. 4
P1011	2	36'-7"	8	31'-1 1/2"	2'-7 1/2"	0'-8"	3'-3 1/2"	315	Top Cap. 4
P1012	1	31'-4"	Str.					195	Bot. Cap. 4
P1013	2	30'-10"	Str.					265	Bot. Cap. 4
P1014	2	30'-0"	Str.					258	Bot. Cap. 4
P1015	10	16'-1"	Str.					692	Column 2
P1016	10	16'-9"	Str.					721	Column 2
P901	68	6'-5"	4	5'-0"	1'-7"			1434	Footings 1, 2, 3 & 4
P902	10	18'-0"	Str.					612	Column 1
P903	10	16'-10"	↑					572	Column 3
P904	2	32'-0"	↑					218	Bot. Cap. 1 & 3
P905	4	31'-6"	↓					428	Bot. Cap. 1 & 3
P906	4	31'-0"	↓					422	Bot. Cap. 1 & 3
P907	4	29'-10"	Str.					406	Bot. Cap. 1 & 3
P908	16	8'-5"	9	2'-10"	2'-9"			458	Bot. Cap. 1, 2, 3 & 4
P909	10	16'-5"	Str.					398	Column 2
P910	14	15'-6"	Str.					738	Column 4
P911	10	15'-10"	Str.					538	Column 4
P912	14	16'-2"	Str.					770	Column 4
P601	120	8'-6"	11	7'-2"				1532	Footings 1 & 3
P602	72	9'-0"	11	7'-8"				973	Footings 2
P603	60	8'-0"	11	6'-8"				721	Footings 4
P501	72	6'-10"	1	2'-8"	2'-3"			513	Caps. 1, 2, 3 & 4
P502	72	7'-6"	1	2'-8"	2'-7"			563	Caps. 1, 2, 3 & 4
P503	16	29'-10"	Str.					498	Caps. 1, 2, 3 & 4
SP401	3	15'-2 3/4"	10	0'-4 1/2"	15'-2 3/4"	44	2'-8"	852	Column 1
SP402	3	13'-7 1/8"	10	0'-4 1/2"	13'-7 1/8"	39	2'-8"	736	Column 3
SP403	3	14'-0 3/8"	10	0'-4 1/2"	14'-0 3/8"	40	2'-8"	774	Column 2
SP404	3	13'-0 3/4"	10	0'-4 1/2"	13'-0 3/4"	38	2'-8"	735	Column 4
SUPERSTRUCTURE									
S701	592	35'-8"	Str.					43158	Slab
S601	72	24'-0"	Str.					2595	Over Piers 1, 2 & 3
S602	24	20'-0"	Str.					721	Over Pier 4
S603	536	30'-0"	Str.					24152	Slab & Curbs
S604	67	21'-3"	Str.					2138	Slab & Curbs
S501	330	2'-3"	1	1'-3"	0'-7 1/2"			774	Curbs
S502	165	3'-6"	1	2'-6"	↑			602	↑
S503	165	2'-2"	1	1'-2"	↓			373	↓
S504	165	5'-6"	1	4'-6"	↓			947	↓
S505	165	2'-4"	1	1'-4"	0'-7 1/2"			402	↓
S506	344	5'-5"	2	2'-1"	0'-8"			1943	Curbs
*R506	112	15'-5"	Str.						Railing
*R507	16	12'-6"	Str.						Railing

NOTES:

- All dimensions are out to out of bar.
 - Radius dimensions are to outside of bar.
 - The length of bent bars is measured along centerline.
 - Bar size is indicated in the bar mark. The first digit where three digits are used, the first two digits where four are used indicates the bar size.
- Four steel channel, tee or angle spacers weighing approximately 0.68 lbs. per lin. ft. of spacer shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantity for spiral bar.
- The "Length" shown in the steel list for the spiral bars is the distance from the top of the footings to the bottom of the pier cap.
- The "No. of Turns" shown is the "Length" divided by the pitch, plus 3 turns (total number of closed coils), expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item 509. 1/2 closed coils shall be provided at the ends of each spiral unit.

MARK	NO.	LENGTH	TYPE
RE501	1	6'-7"	Str.
RE601	2	6'-11"	↑
RE701	3	7'-3"	↑
RE801	1	7'-6"	↑
RE901	1	7'-10"	↑
RE1001	1	8'-3"	↓
RE1101	1	8'-7"	Str.

*Included with Item 517 for payment

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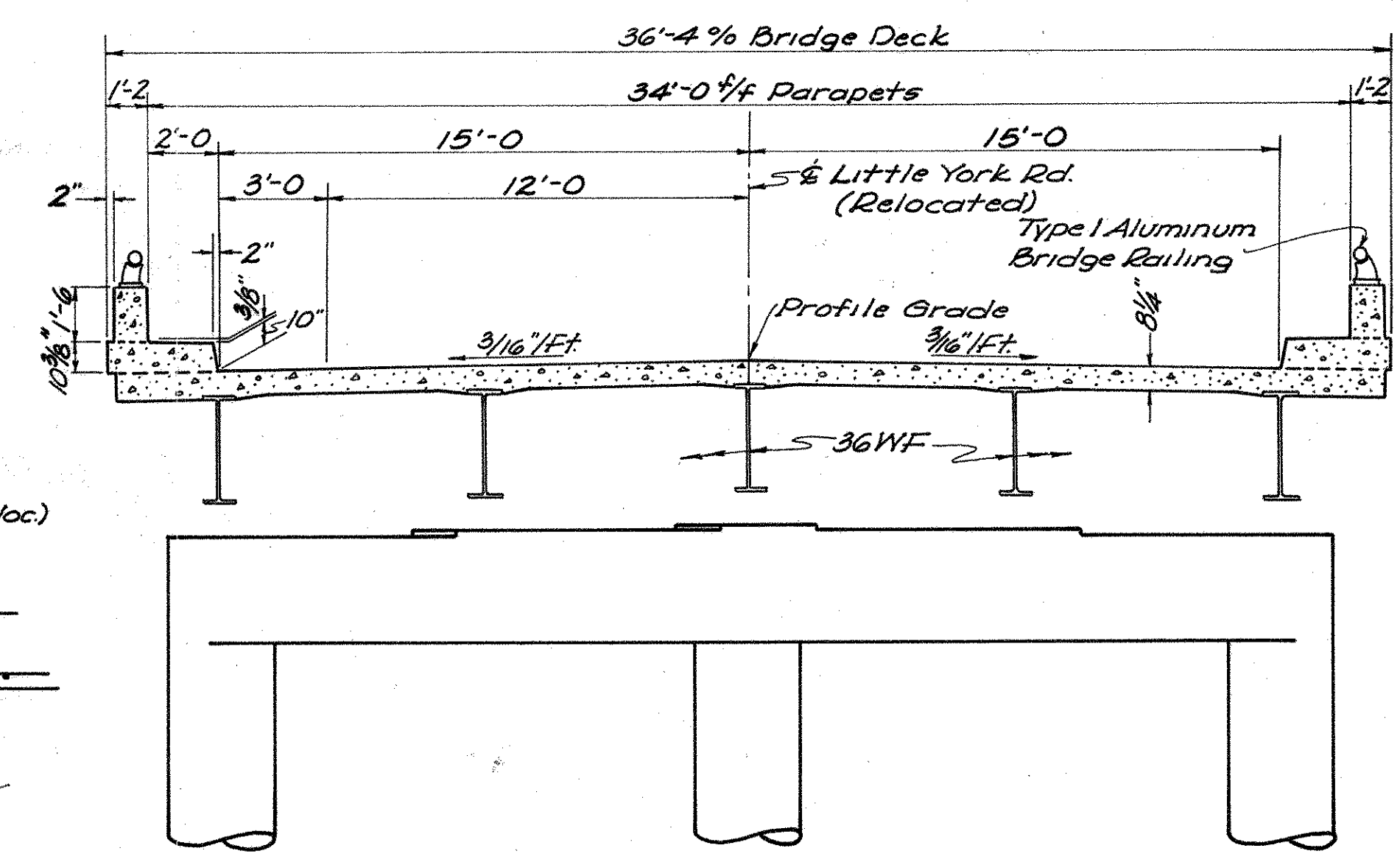
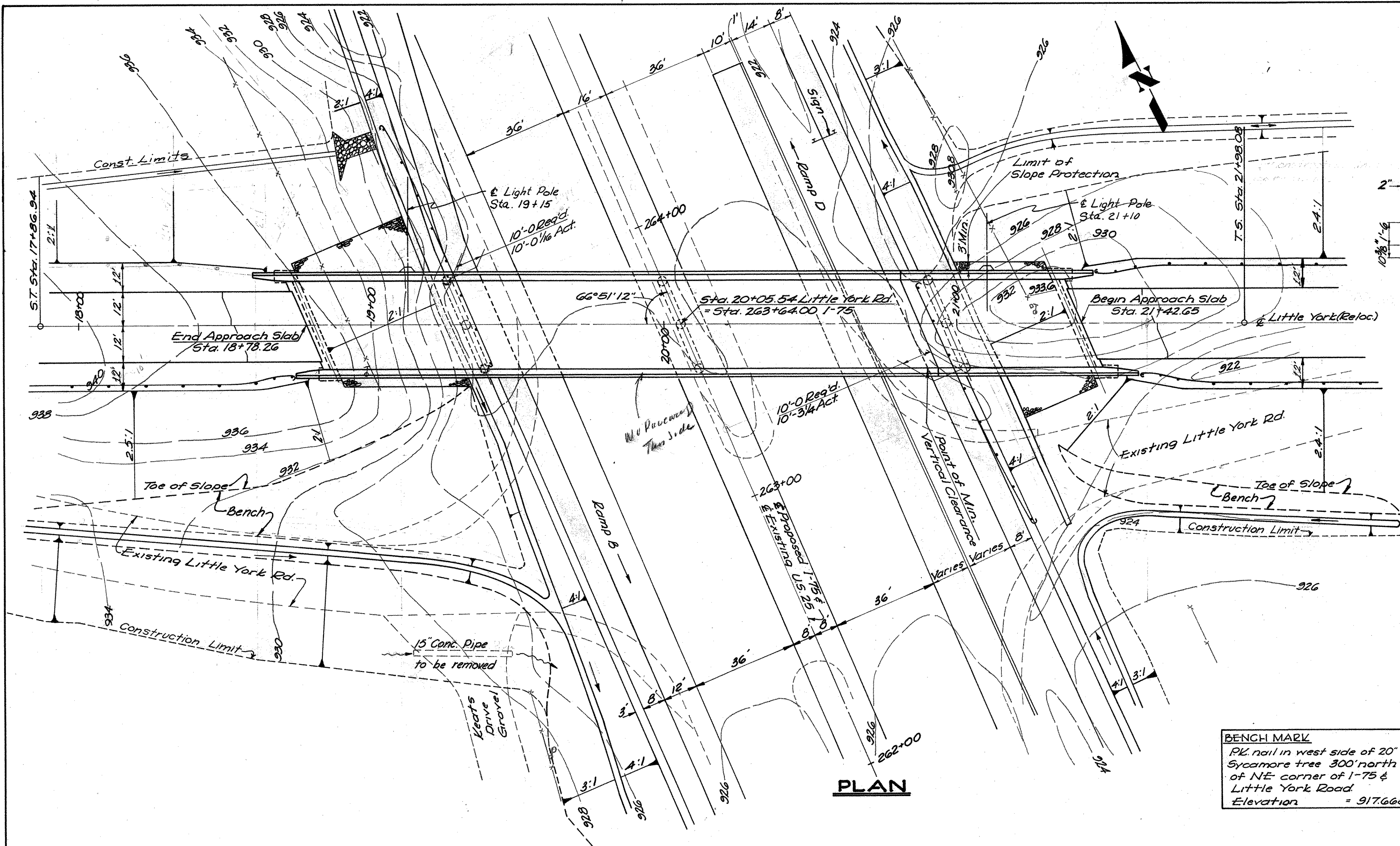
CINCINNATI OHIO

REINFORCING STEEL DETAILS

BRIDGE NO. MOT. 75-1579
STOP EIGHT OVER I-75

MONTGOMERY CO. STA. 135+82.49

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	W.B.S.	L.R.F.	R.J.L.	12-29-67	



SECTION - BRIDGE DECK

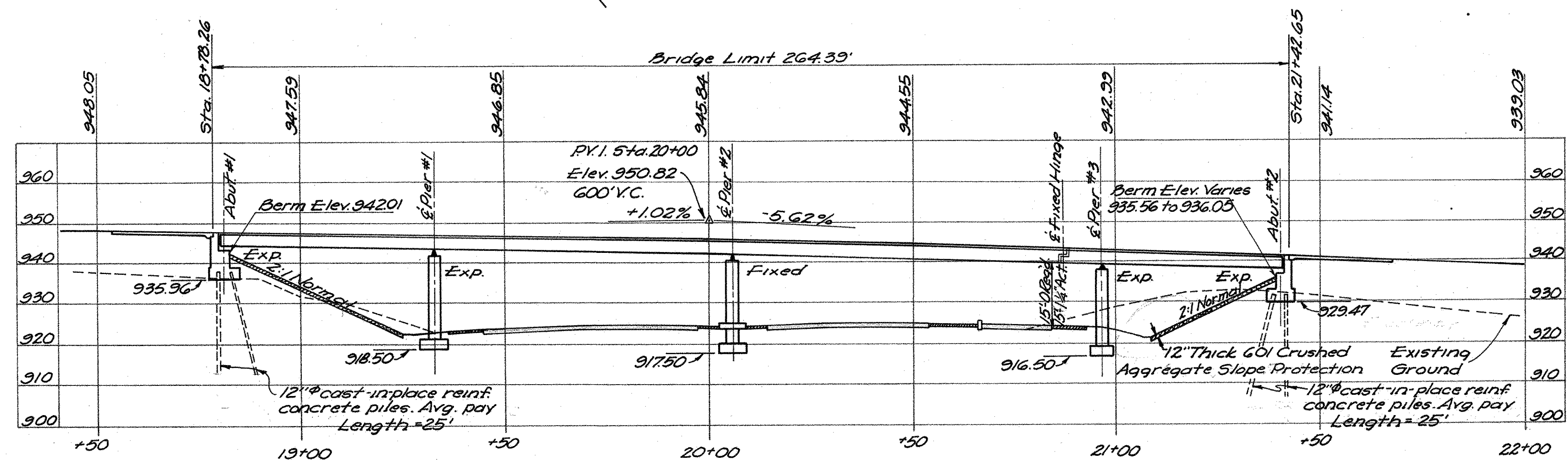
AVERAGE DAILY TRAFFIC

A.D.T. (1975)	2540
TYPE "B" (Estimated)	160

PROPOSED STRUCTURE

TYPE: Continuous steel beam with reinforced concrete deck and substructure.
 SPANS: 51'-5", 73'-5", 30'-8" and 44'-0"
 ROADWAY: 30'-0" of curbs, with 2'-2" safety curbs.
 LOAD FREQUENCY: CF=400(57)
 SKEW: 23°-08'-48" Rt. Forward
 WEARING SURFACE: Monolithic concrete.
 APPROACH SLABS: A5-1-67 (25' long)
 ALIGNMENT: Tangent

BENCH MARK
 PK nail in west side of 20' Sycamore tree 300' north of NE corner of 1-75 & Little York Road.
 Elevation = 917.668



PROFILE ALONG &

SHAW, LENZ & ASSOCIATES ENGINEERS
 CINCINNATI OHIO

SITE PLAN
 BRIDGE NO. MOT-75-2127
 LITTLE YORK RD over I-75

MONTGOMERY COUNTY STA. 263+44.24

Surveyor	Drawn	Designed	Drawn	Checked	Reviewed
L.Shad	W.T.B.	W.B.S.	W.T.B.	E.J.L.	

ESTIMATED QUANTITIES							
ITEM	TOTAL	UNIT	DESCRIPTION	ABUTS.	PIERS	SUPER-STRUCTURE	GENERAL
503	403	Cu.Yds.	Unclassified Excavation	197	206		
505	Lump	Lump Sum	First Test Pile	Lump			
507	600	Lin.Ft.	12" ϕ Cast-in-place reinforced concrete piles	600			
509	123,361	Lbs.	Reinforcing Steel	9,775	35,262	78,524	
511	298	Cu.Yds.	Class "C" Concrete, superstructure			298	
511	82	Cu.Yds.	Class "C" Concrete, piers above footings		82		
511	91	Cu.Yds.	Class "C" Concrete, abutments above footings	91			
511	152	Cu.Yds.	Class "C" Concrete, footings	67	85		
513	278,055	Lbs.	Structural Steel			278,055	
516	39	Lin.Ft.	Preformed elastic joint sealer (705.11)			39	
517	578	Lin.Ft.	Bridge Railing, Type 1	56		522	
518	29	Cu.Yds.	Porous Backfill	29			
518	60	Lin.Ft.	6" Perforated helical C.M.P. including specials 707.01	60			
518	53	Lin.Ft.	6" Non-perforated helical C.M.P. 707.01	53			
518	10	Each	Scuppers (including supports)			10	
601	455	Sq.Yds.	Crushed Aggregate Slope Protection				455
808	298	Units	Water-reducing set retarding admixture			298	
514	278,055	Lbs.	Field Painting of Structural Steel			278,055	
625			See lighting plans.				

GENERAL NOTES

REFERENCE shall be made to the following:

Standard Drawings: BR-1-65 Sheet 1 of 2 revised 11-24-65
 RB-1-55 Revised 2-2-59
 SD-1-65 Sheets 1, 2 & 3, dated 11-8-65
 AS-1-67 Dated 1-11-68

Supplemental Specifications: 811, and 808 dated 1-1-69,
 and 836 dated 6-17-69

DESIGN SPECIFICATIONS This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways dated 9-1-57 together with current revisions thereof

DESIGN LOADING CF-400 (BT)

CONCRETE Class "C" basic unit stress = 1333 p.s.i. for superstructure
 Class "C" basic unit stress = 1133 p.s.i. for substructure

STRUCTURAL STEEL ASTM A-36 - basic unit stress = 20,000 p.s.i.

WELDS on Non-stress carrying members are shown thus:

JOINT-SEALER: Item 828 joint sealer including bond breaker, shown in Section A-A of Std. Dwg. SD-1-65, sheet No.1, shall be omitted.

REINFORCING STEEL

ASTM, A615, A616, A617, Deformed Intermediate or Hard. Basic unit stress: 20,000 p.s.i. Spiral reinforcement shall be plain, A306 or A499

PILES shall be driven to a minimum bearing capacity of 30 tons per pile for the abutments.

FOUNDATION BEARING PRESSURE Pier footings are designed for a maximum bearing pressure of 2.5 tons per sq. ft.

MACHINE FINISH

The concrete deck shall be finished by the use of a finishing machine.

UTILITY LINES

All expenses involved in relocating the affected utility lines shall be borne by the owners. The Contractors and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either is held to a minimum.

EXCAVATION QUANTITY includes the removal of fill materials required for construction of the abutments.

WELDED ATTACHMENTS

No attachments shall be made by welding to the top flanges of the beams within a distance of 1/10 of the span length on either side of the interior supports. Welding for attachments to the top flanges at other parts of the spans shall be kept at least 2" from edge of flange.

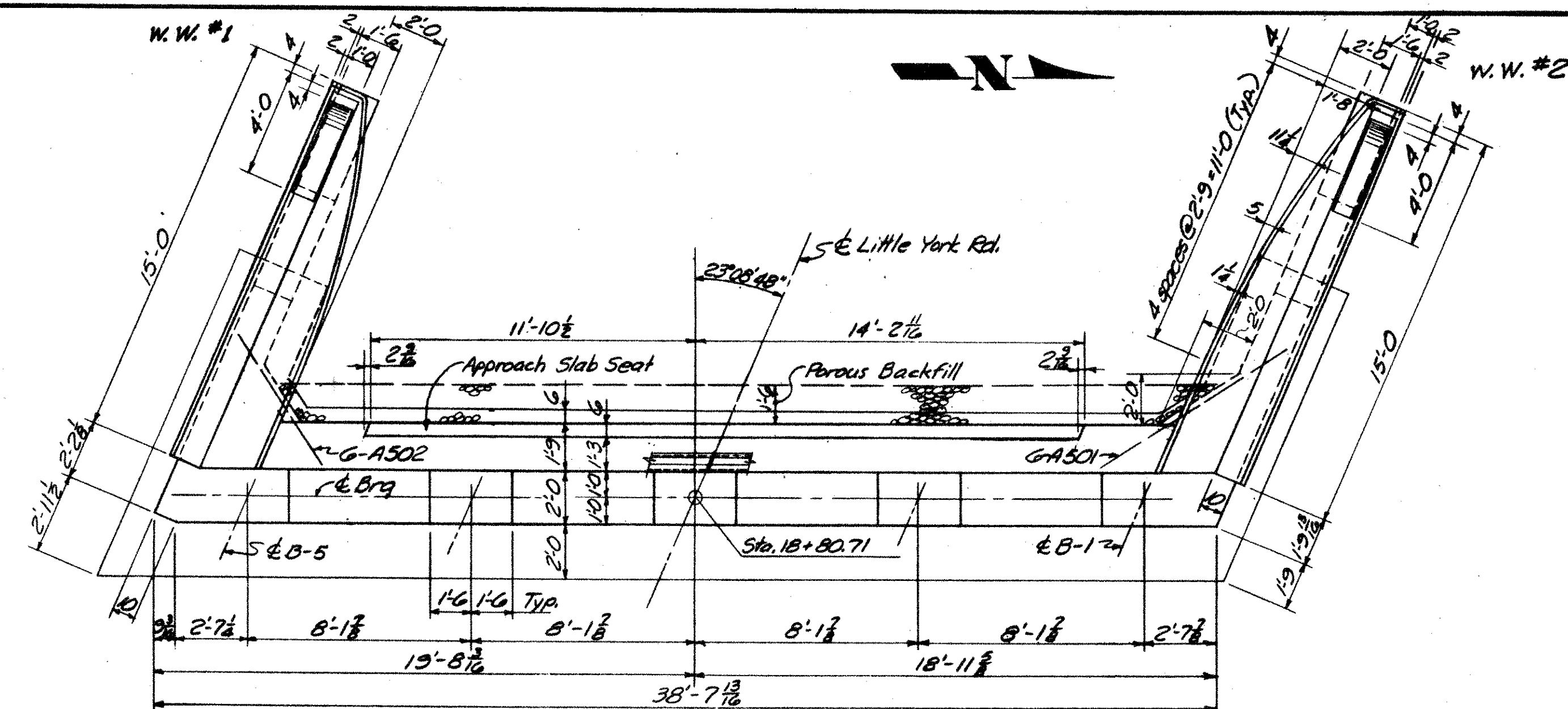
SCUPPERS including support angles, shall be galvanized in accordance with 711.02

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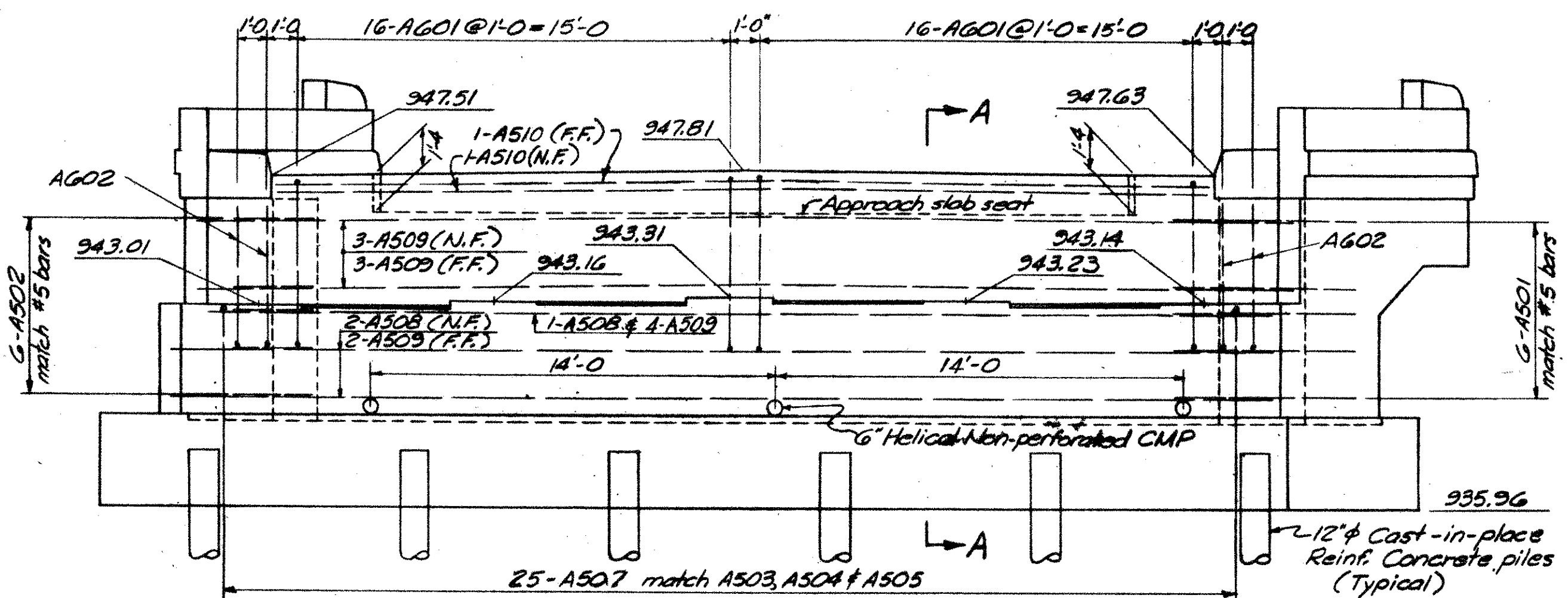
QUANTITIES & GENERAL NOTES

BRIDGE NO. MOT-75-2127
 LITTLE YORK OVER I-75

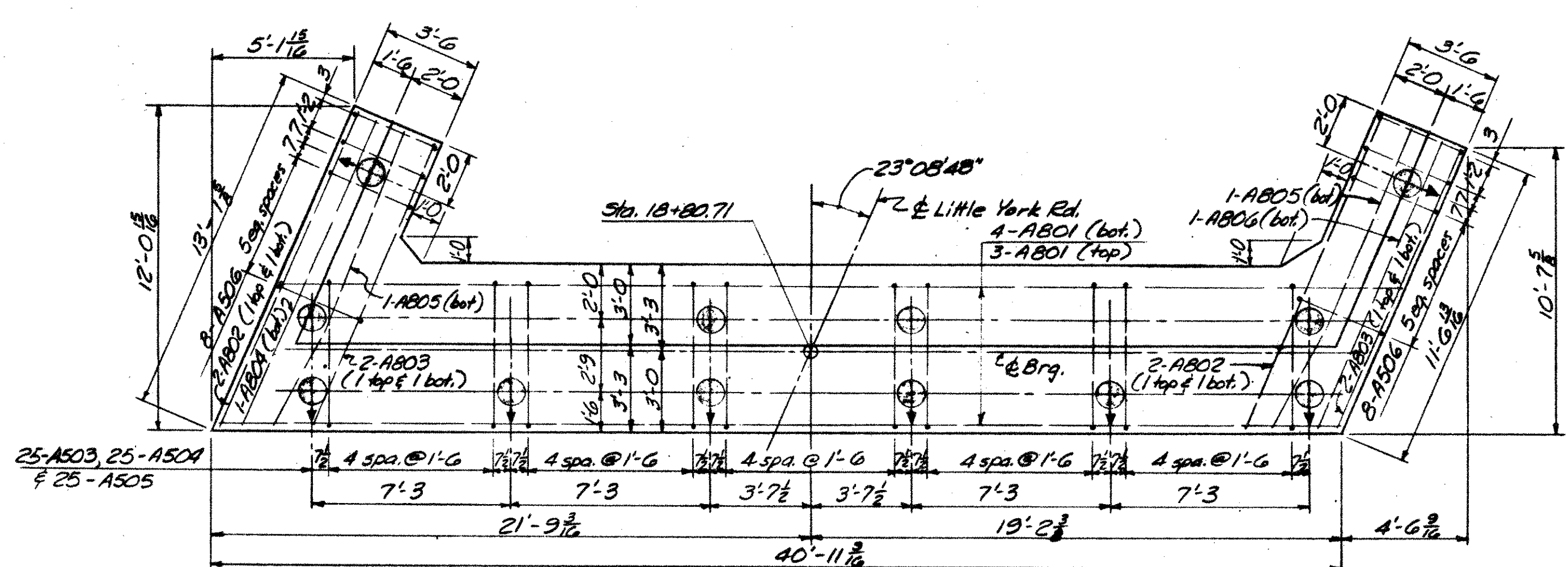
MONTGOMERY CO. STA. 203+44.24
 DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
 W.B.S. T.L.U. R.J.L.



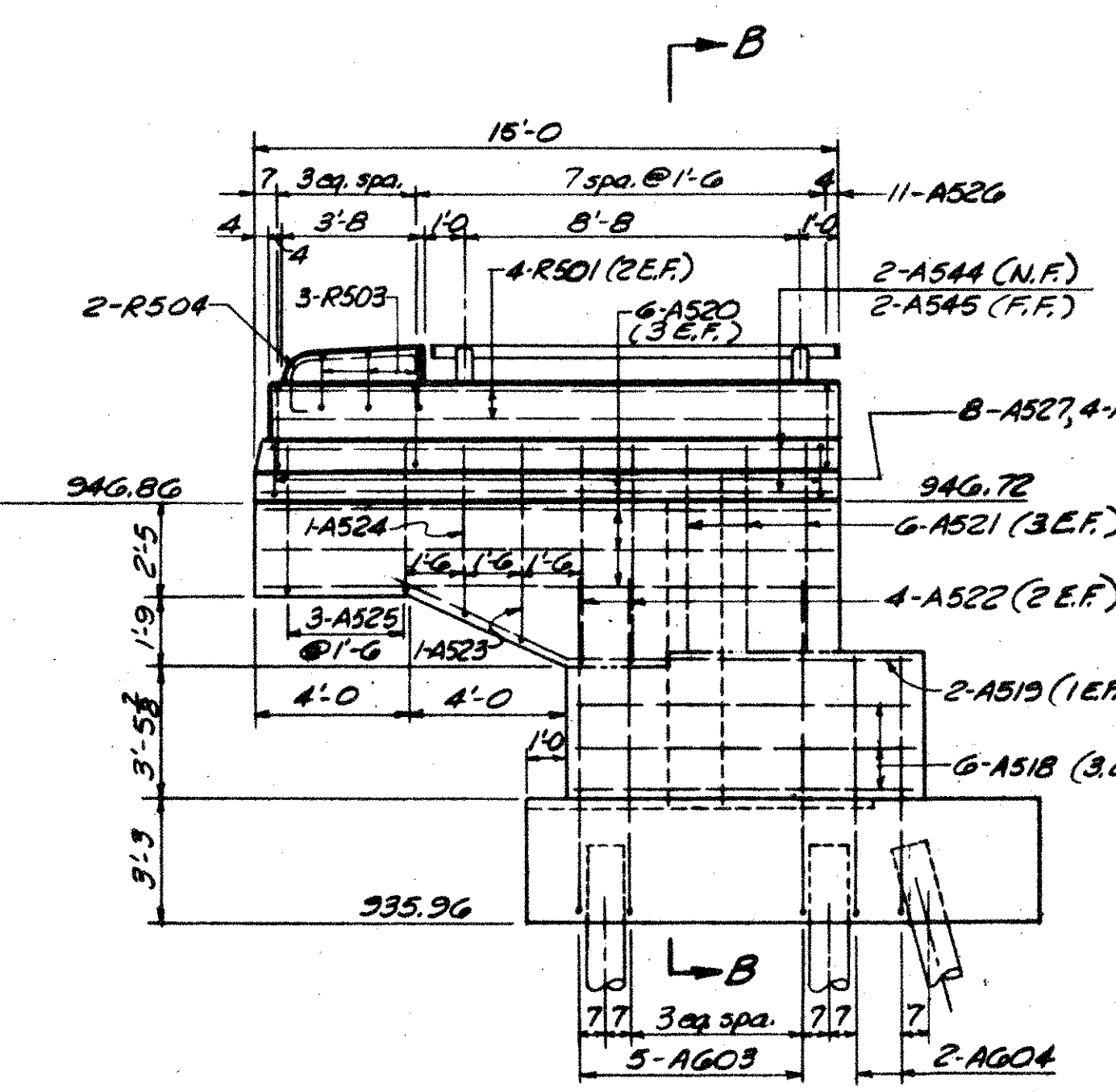
PLAN



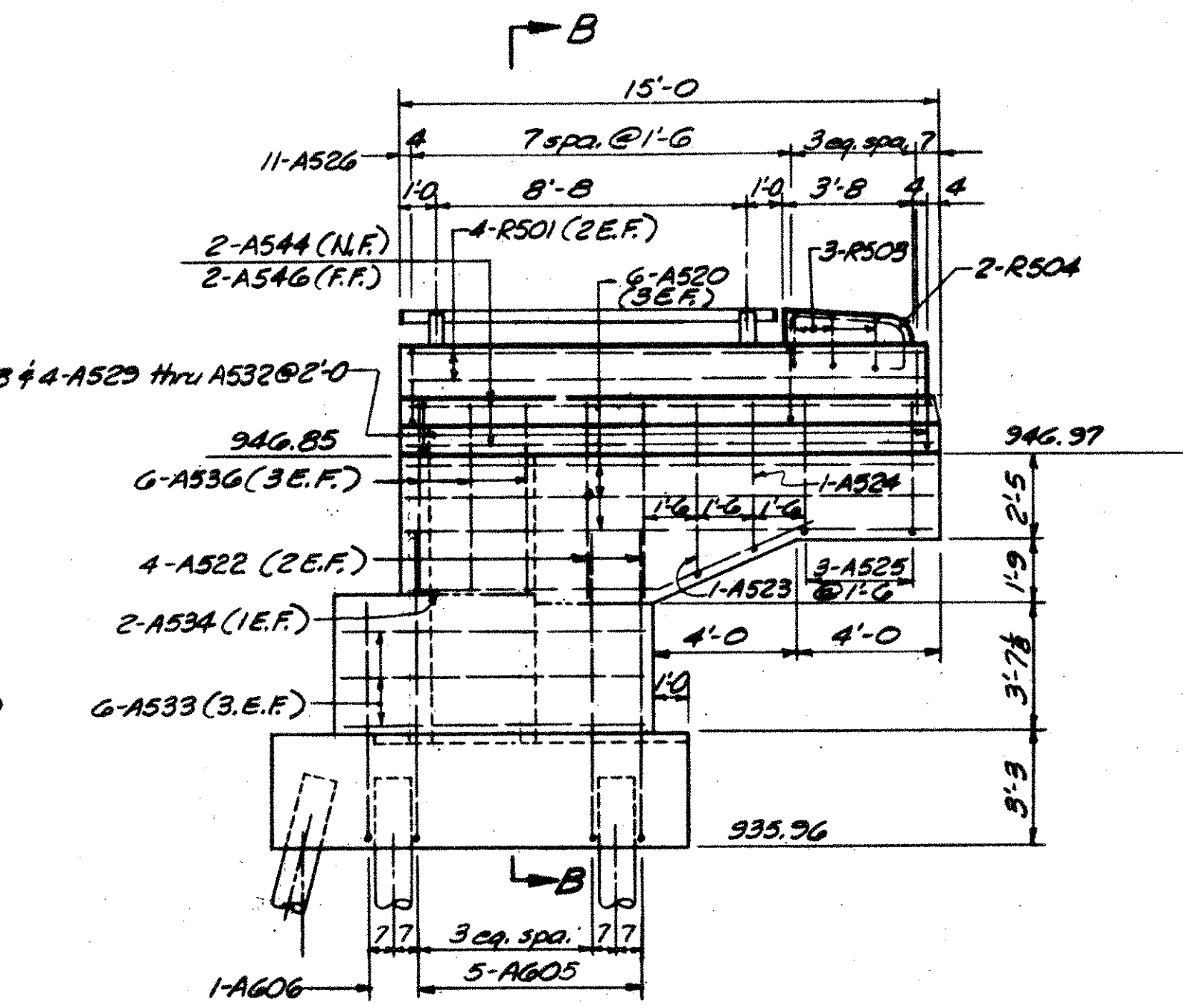
ELEVATION

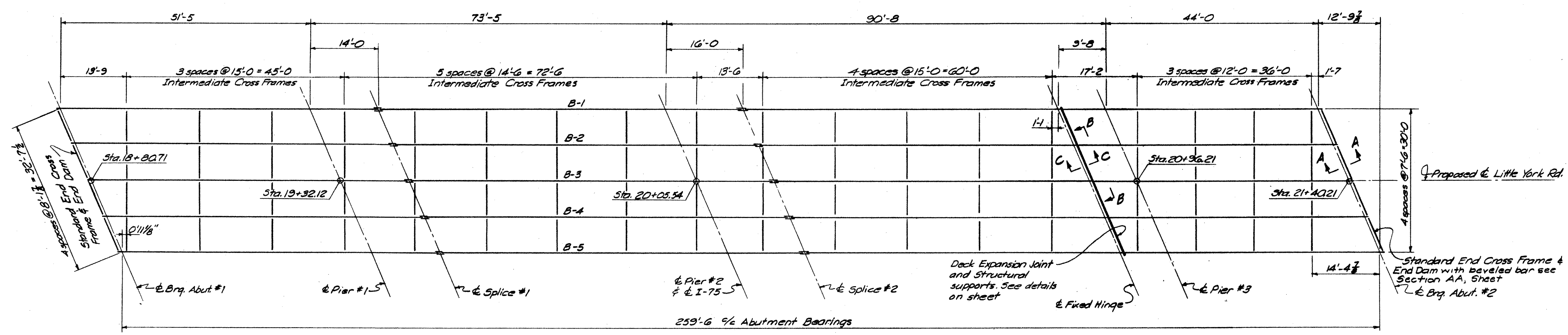
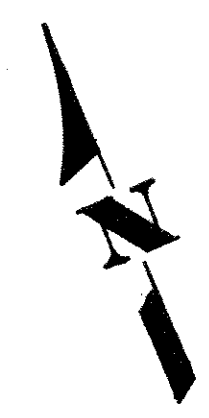


FOOTING PLAN

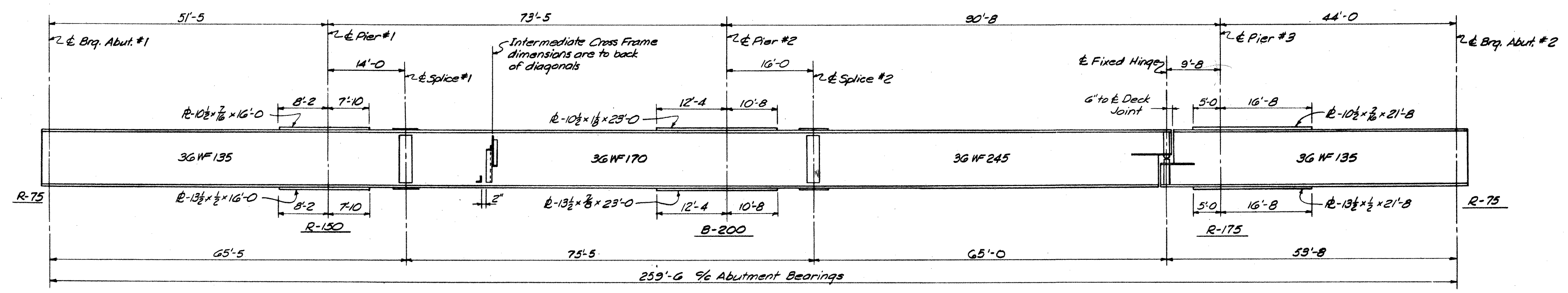


ELEVATION - WINGWALL #1

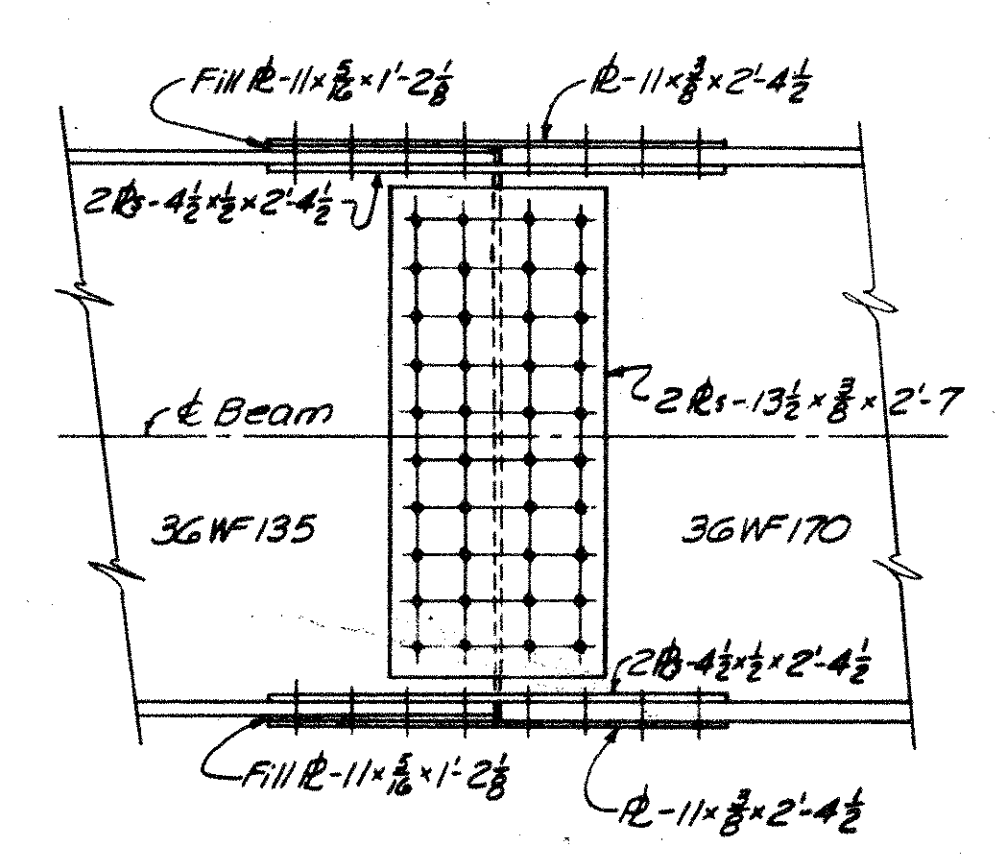




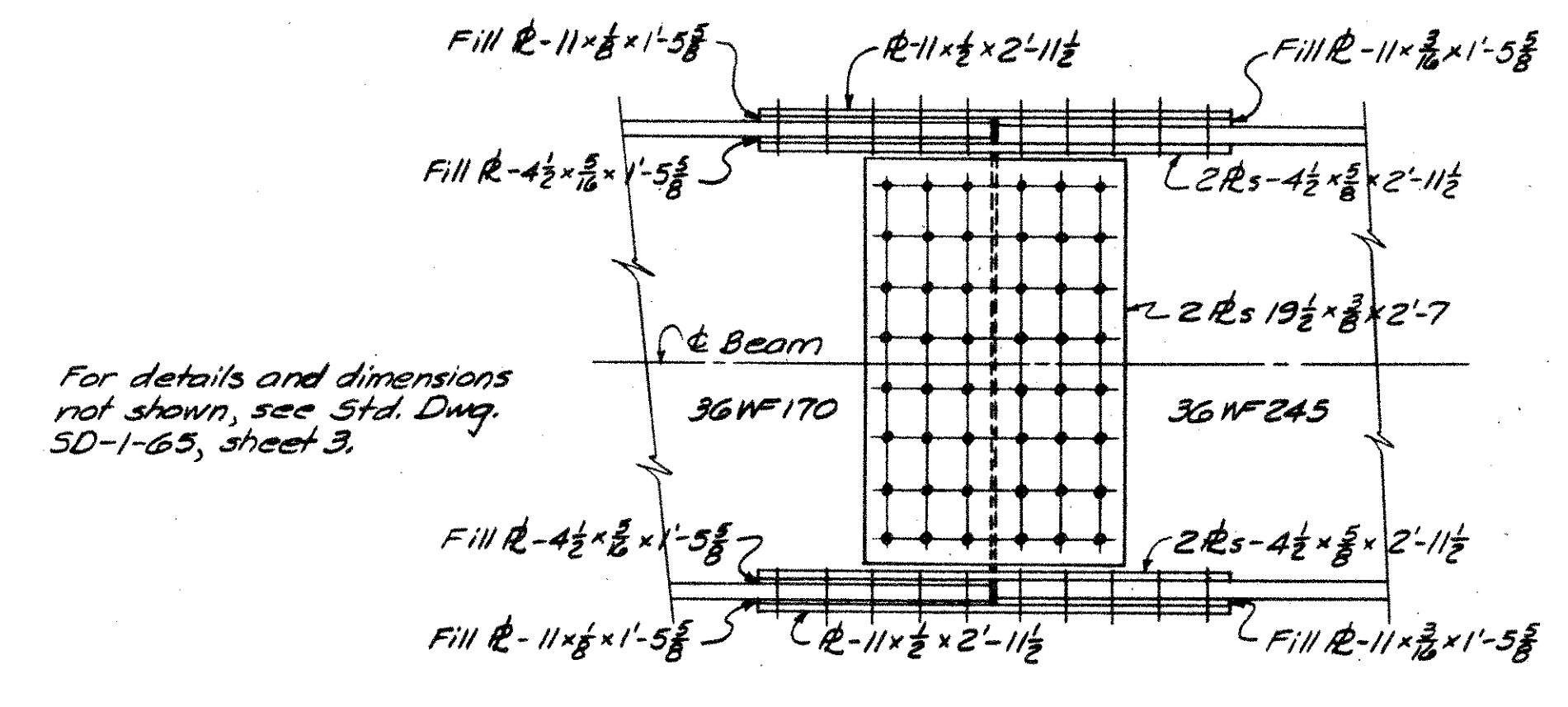
FRAMING PLAN



TYPICAL BEAM ELEVATION



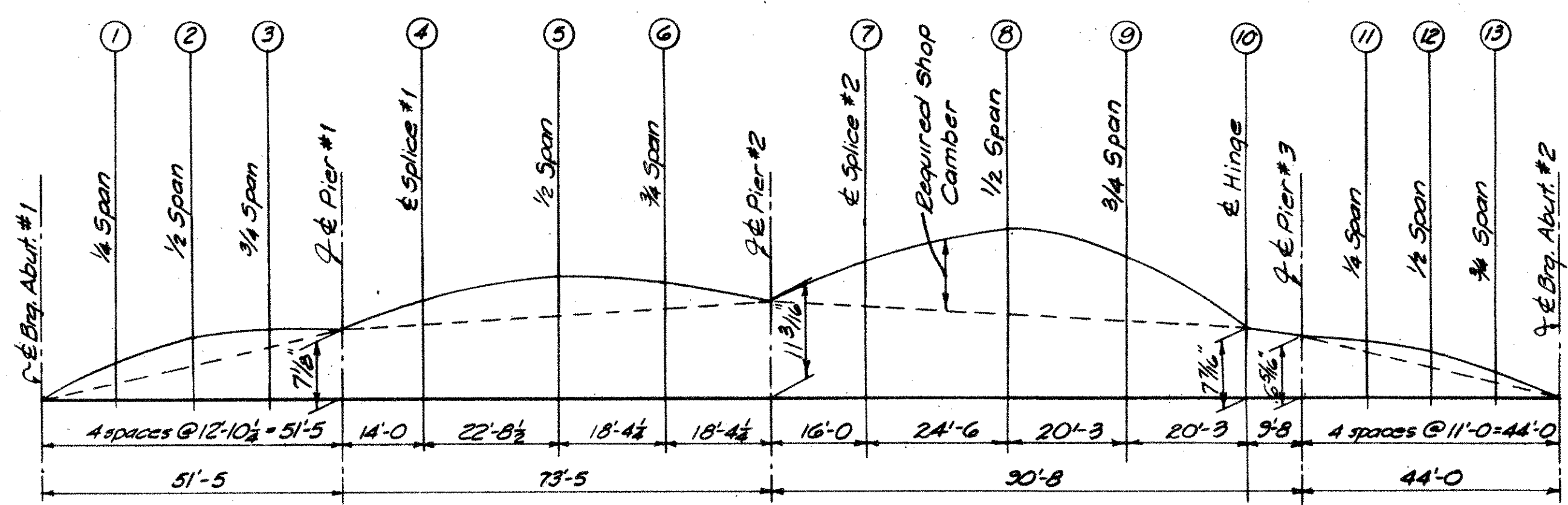
DETAIL - SPLICE #1



DETAIL SPLICE #2

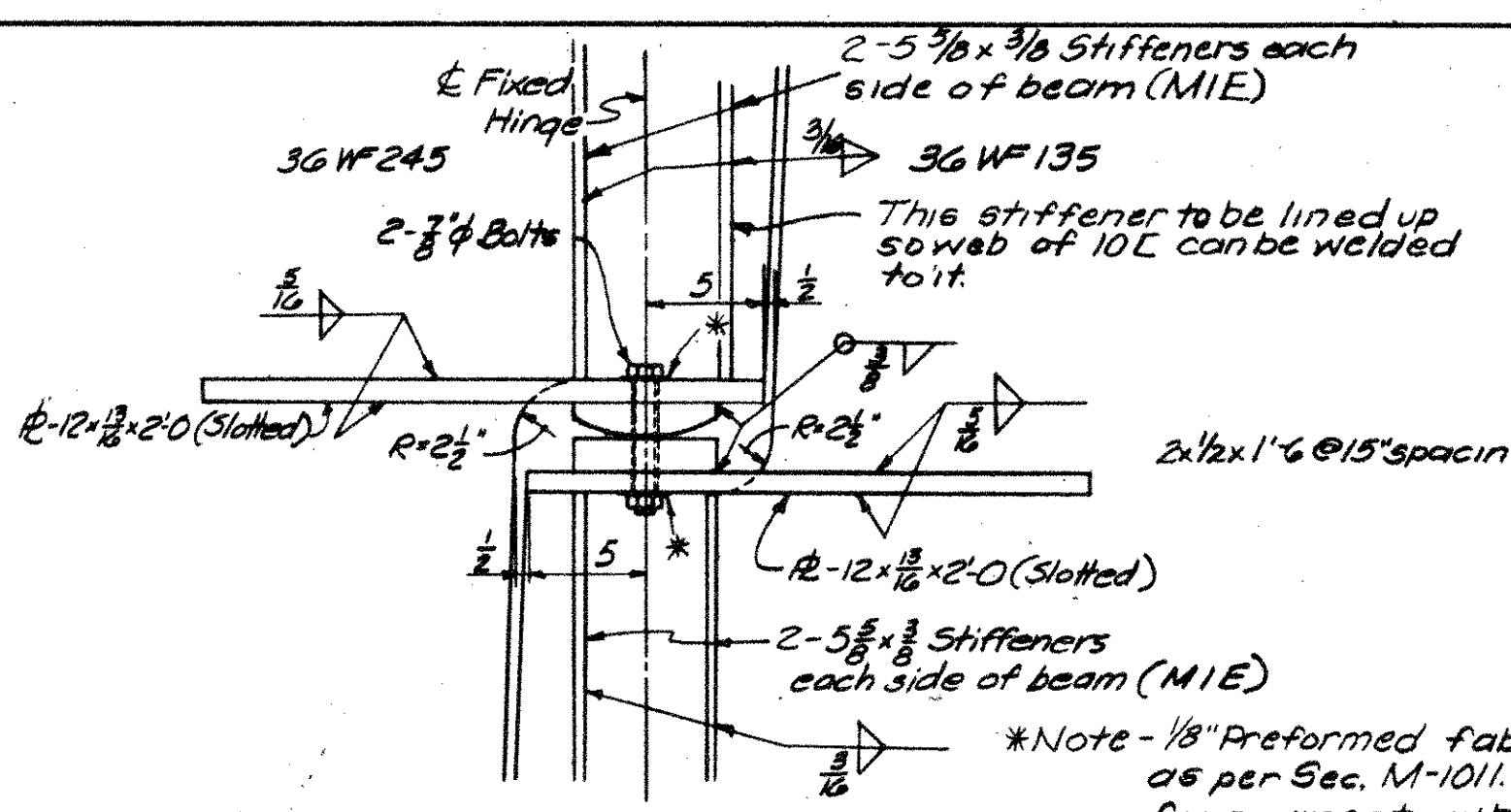
- NOTES:**
- For details at fixed hinge see sheet 7.
 - For beam camber and layout see sheet 7.
 - For intermediate cross frames see sheet 8.
 - For Rockers & Bolsters see Std. Dwg. RB-1-55.
 - For shop welding of moment plates, see Std. Dwg. SD-1-65, sheet 2.
 - For Std. end dam & end cross frame details see Std. Dwg. SD-1-65, sheet 1.
 - For curb plate details, see Std. Dwg. SD-1-65, sheet 2.
 - For additional splice details, see Std. Dwg. SD-1-65, sheet 3.
 - For location of scuppers, see sheet 8.

SHAW, LENZ & ASSOCIATES ENGINEERS				6/10
CINCINNATI				OHIO
FRAMING PLAN				
BRIDGE NO. MOT-75-2127 LITTLE YORK RD. OVER I-75				
MONTGOMERY CO.				STA. 268+44.24
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
W.B.S.	R.R.L.		R.J.L.	

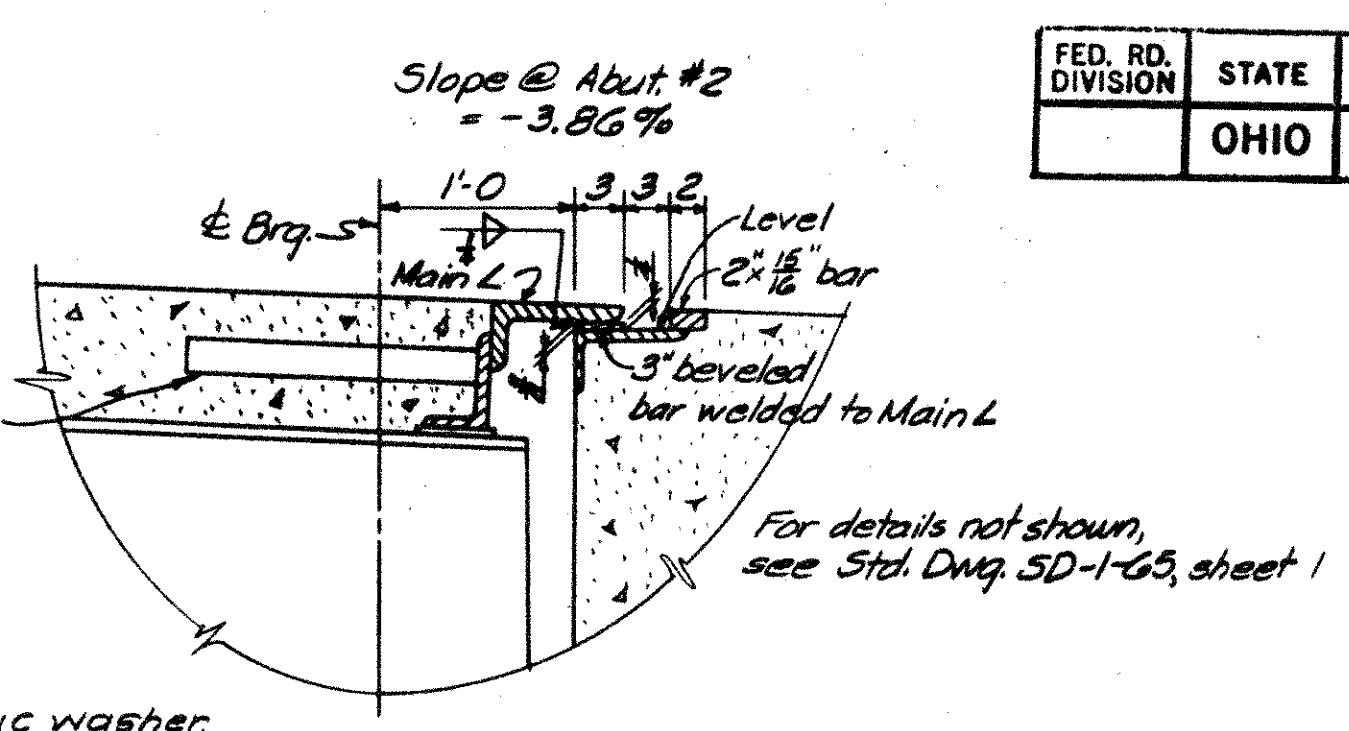


DEFLECTION & CAMBER DIAGRAM

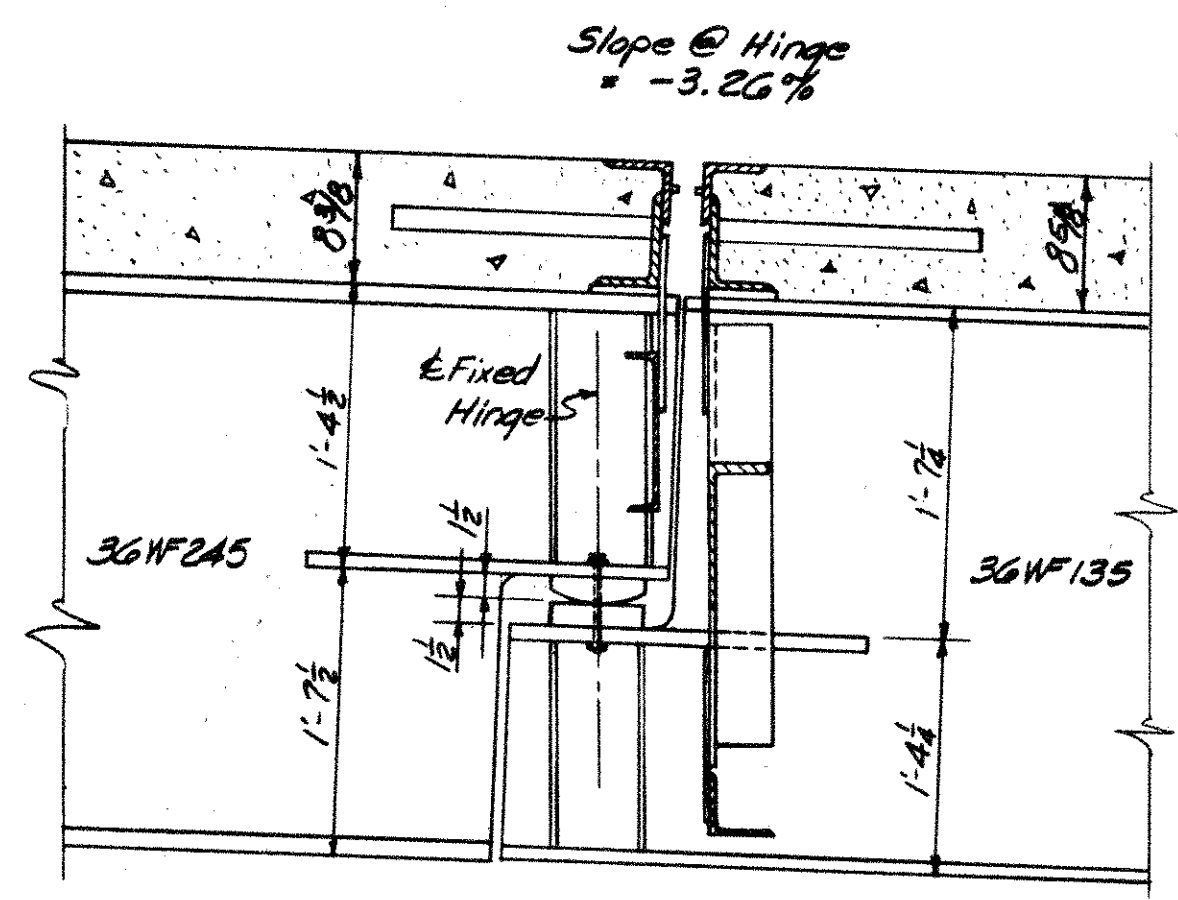
	1	2	3	4	5	6	7	8	9	10	11	12	13
Interior Beam													
Deflection due to D.L. beam	1/32	1/32	1/32	0	0	-1/32	1/8	1/4	1/8	0	0	0	0
Deflection due to remaining D.L.	3/16	3/16	3/32	1/16	5/32	-1/32	5/16	25/32	5/8	0	0	1/16	1/16
Vertical curve adjustment	11/32	15/32	5/16	9/16	7/8	11/16	11/16	13/32	13/16	0	1/4	5/16	1/4
Shop camber required	9/16	11/16	7/16	5/8	1 1/8	5/8	1 1/8	2 1/8	1 3/8	0	1/4	3/8	5/16
Exterior Beam													
Deflection due to D.L. beam	1/32	1/32	1/32	0	0	-1/16	1/8	1/4	1/8	0	0	0	0
Deflection due to remaining D.L.	3/16	1/4	3/32	1/16	3/16	0	3/8	23/32	11/16	0	0	1/16	1/16
Vertical curve adjustment	11/32	15/32	5/16	9/16	7/8	11/16	11/16	13/32	13/16	0	1/4	5/16	1/4
Shop camber required	9/16	3/4	7/16	5/8	1 1/8	5/8	1 3/16	2 1/4	1 5/8	0	1/4	3/8	5/16



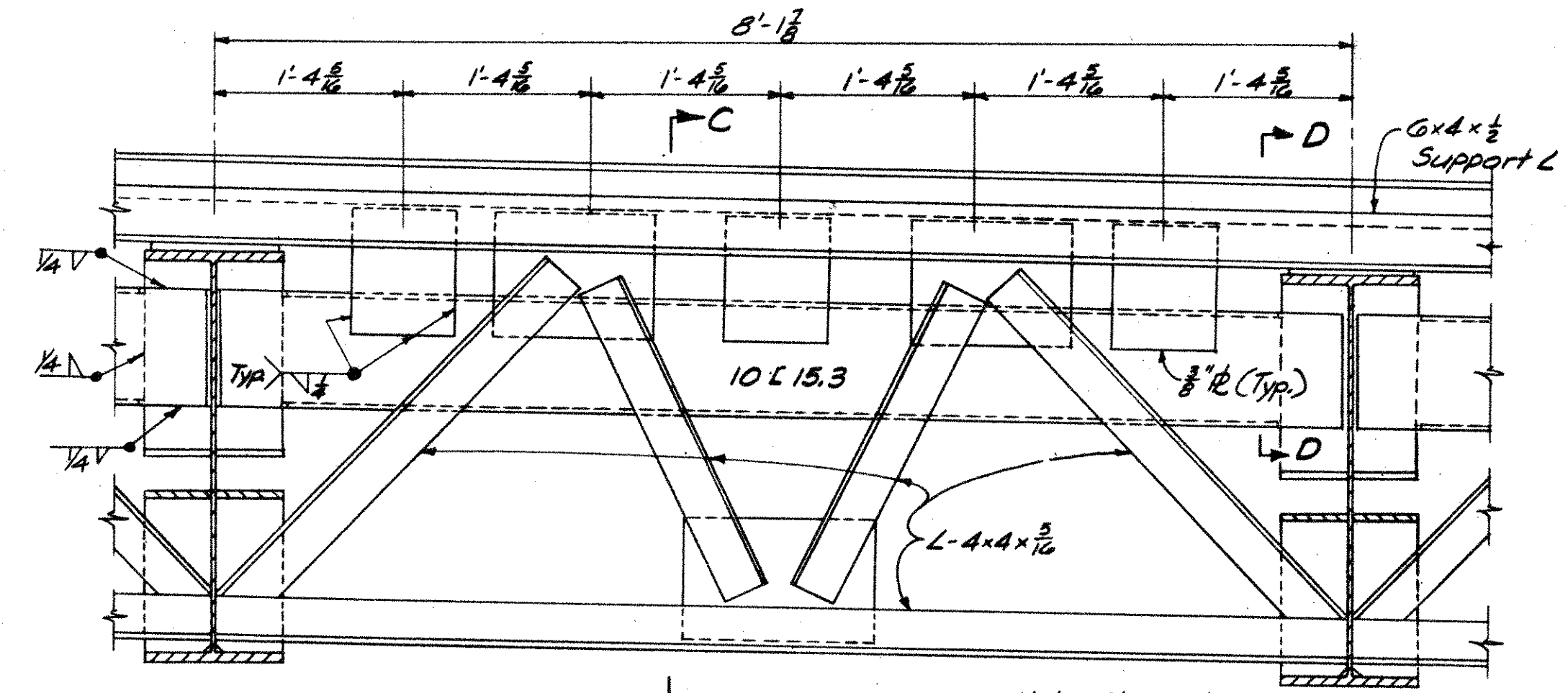
HINGE DETAIL



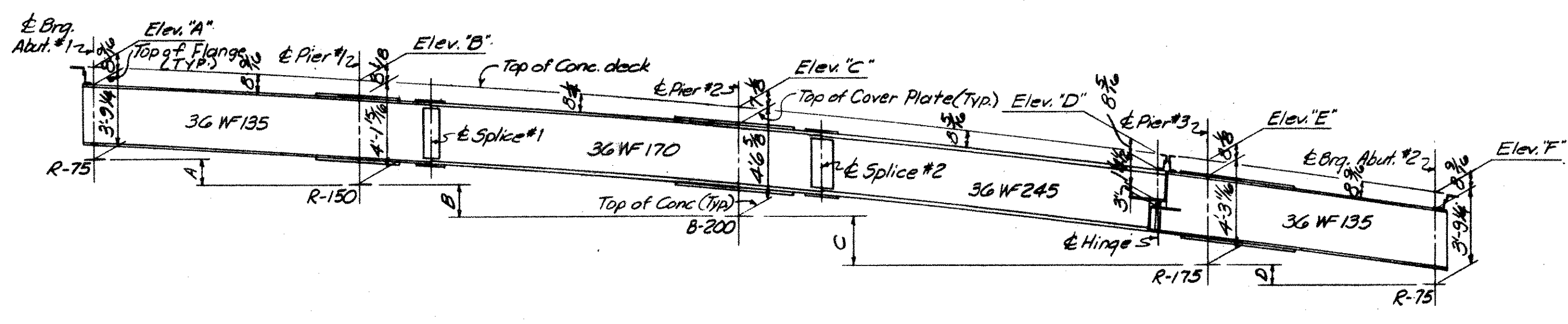
SECTION A-A



SECTION C-C

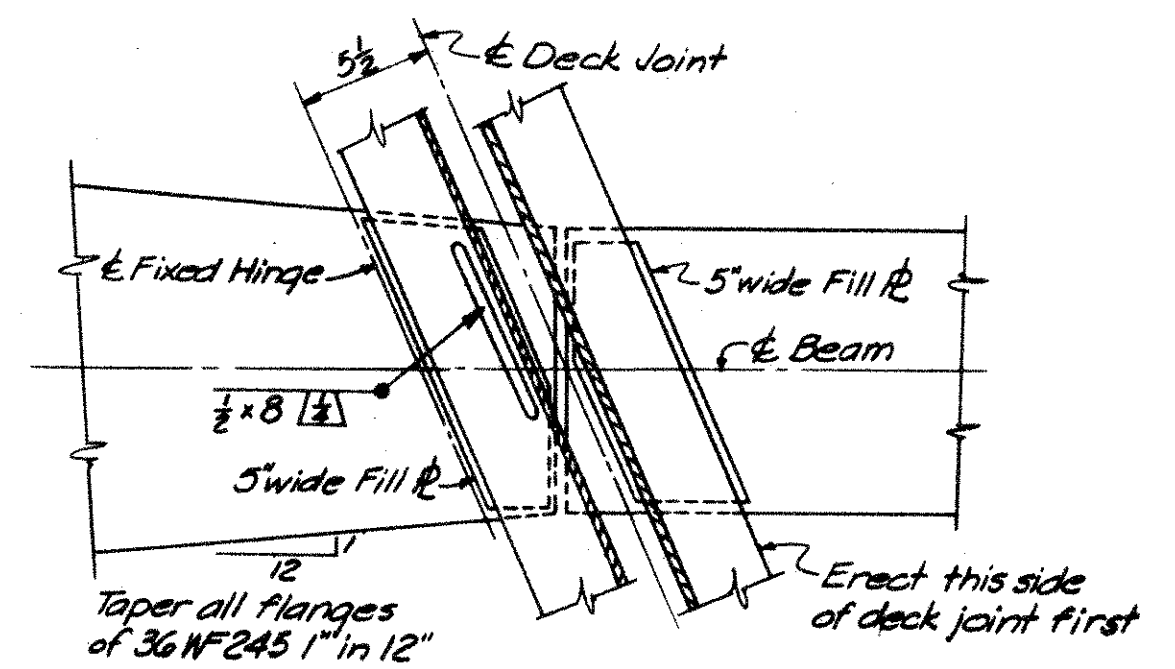


SECTION B-B

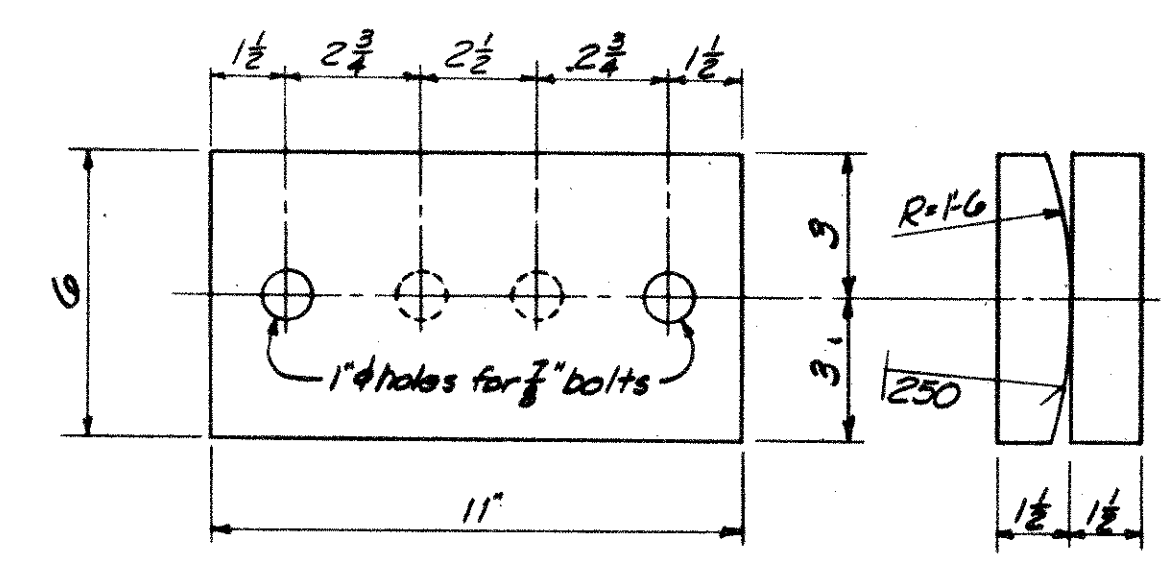


ELEVATION LAYOUT

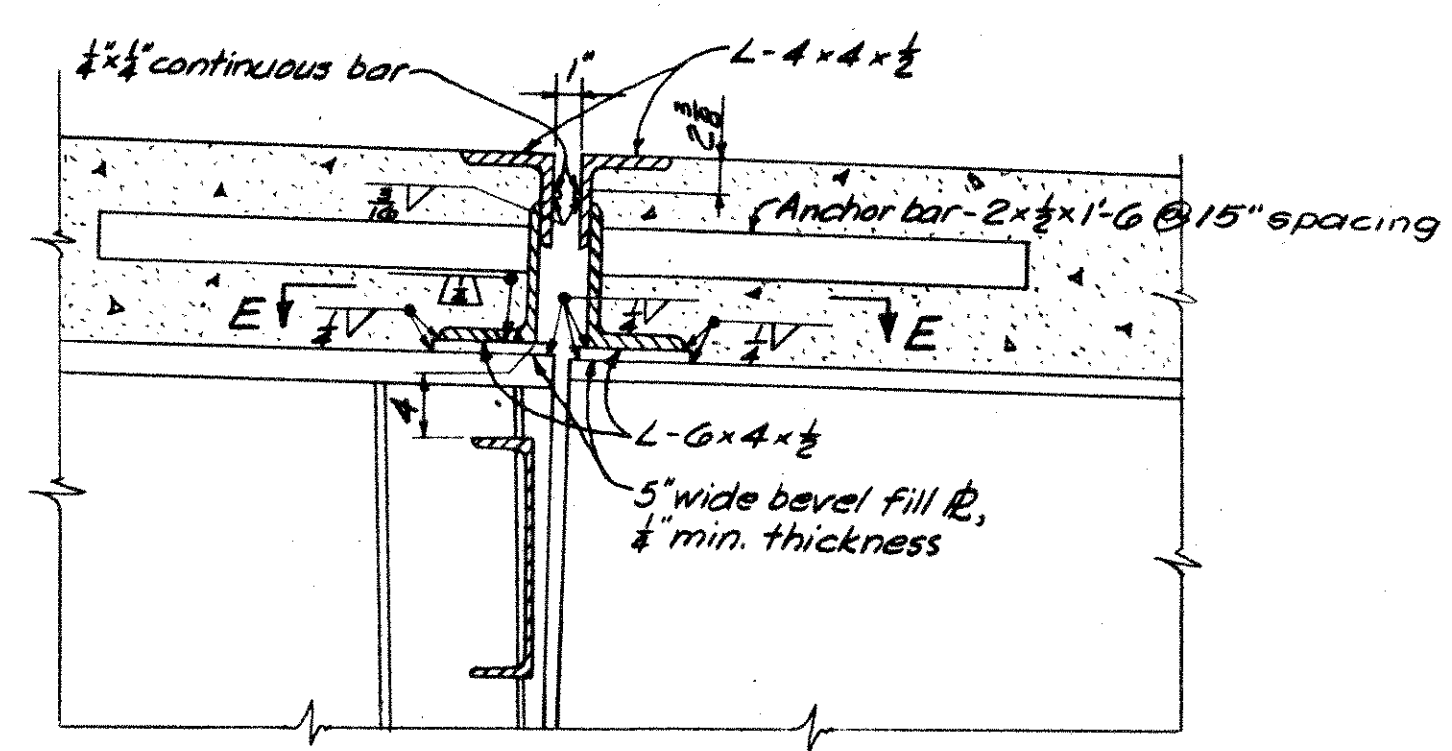
Beam	Elev. "A"	Elev. "B"	Elev. "C"	Elev. "D"	Elev. "E"	Elev. "F"	Dim. "A"	Dim. "B"	Dim. "C"	Dim. "D"
B-1	947.62	947.01	945.63	943.41	943.09	941.54	0'-11 5/8"	1'-8 3/8"	2'-4 3/8"	1'-0 1/8"
B-2	947.71	947.08	945.67	943.42	943.11	941.53	0'-11 13/16"	1'-8 3/8"	2'-4 13/16"	1'-0 7/8"
B-3	947.80	947.15	945.71	943.44	943.12	941.53	1'-0 1/2"	1'-8 3/8"	2'-5 1/4"	1'-1 1/8"
B-4	947.65	946.98	945.52	943.21	942.89	941.29	1'-0 1/2"	1'-9 1/4"	2'-5 5/8"	1'-1 1/4"
B-5	947.50	946.81	945.32	942.99	942.66	941.04	1'-0 1/2"	1'-9 3/8"	2'-6"	1'-1 7/8"



SECTION E-E



HINGE SHOE DETAILS



SECTION D-D

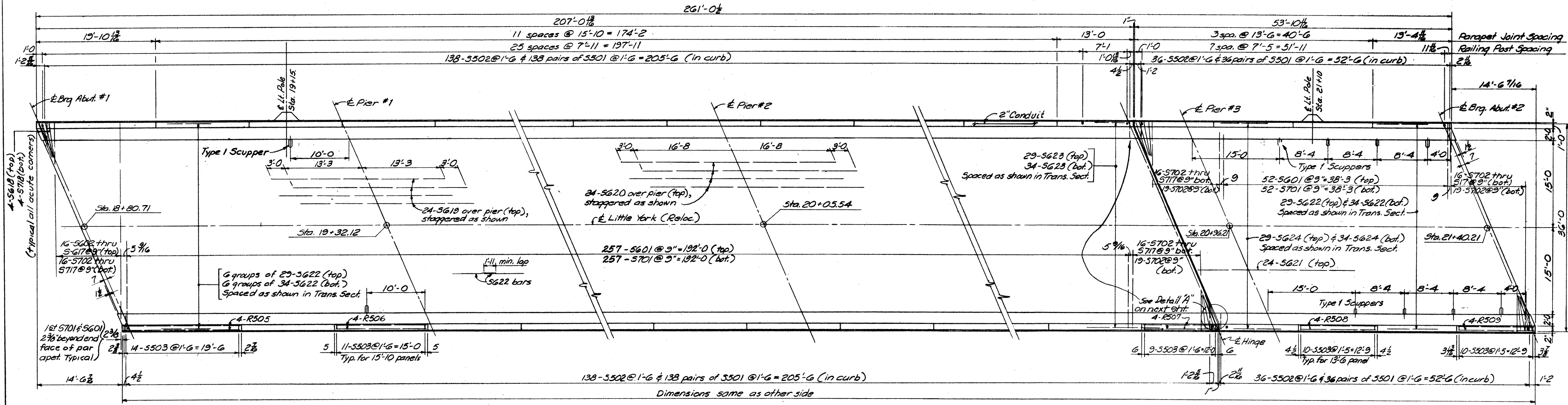
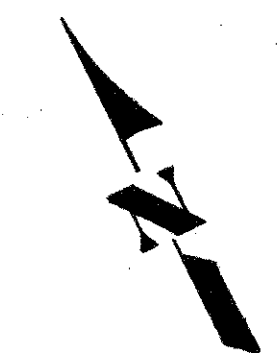
For details not shown, see Std. Dwg. 50-1-65, sheet 1.
For extent of joint angles, see sheet 9.

SHAW, LENZ & ASSOCIATES
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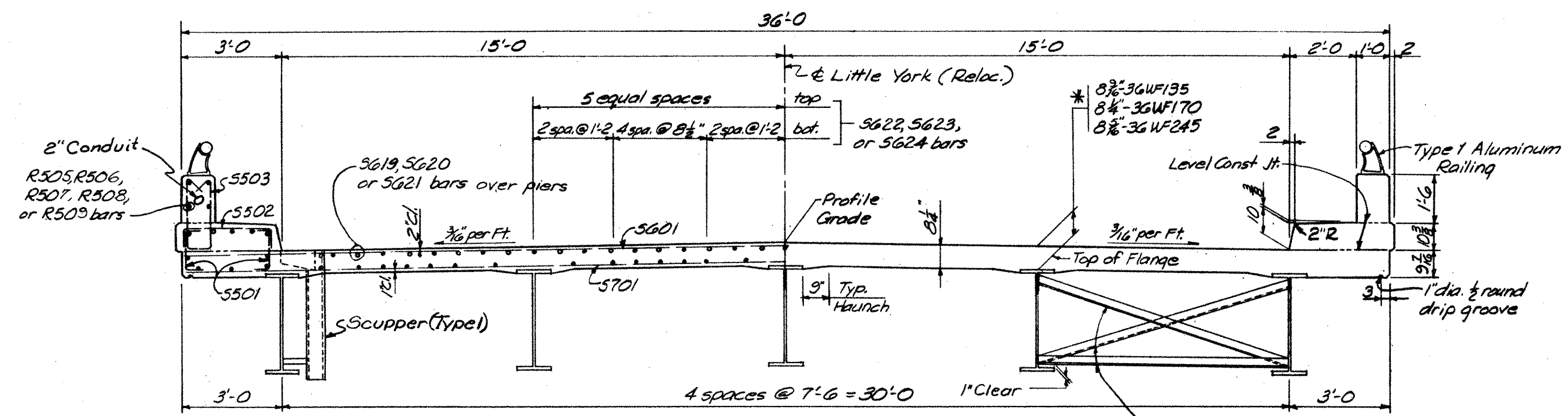
FRAMING PLAN DETAILS
BRIDGE NO. MOT. 75-2127
LITTLE YORK RD. OVER I-75
1-75

MONTGOMERY CO. STA. 268+44.24

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
W.B.S.	R.R.L.		R.J.L.		



PLAN



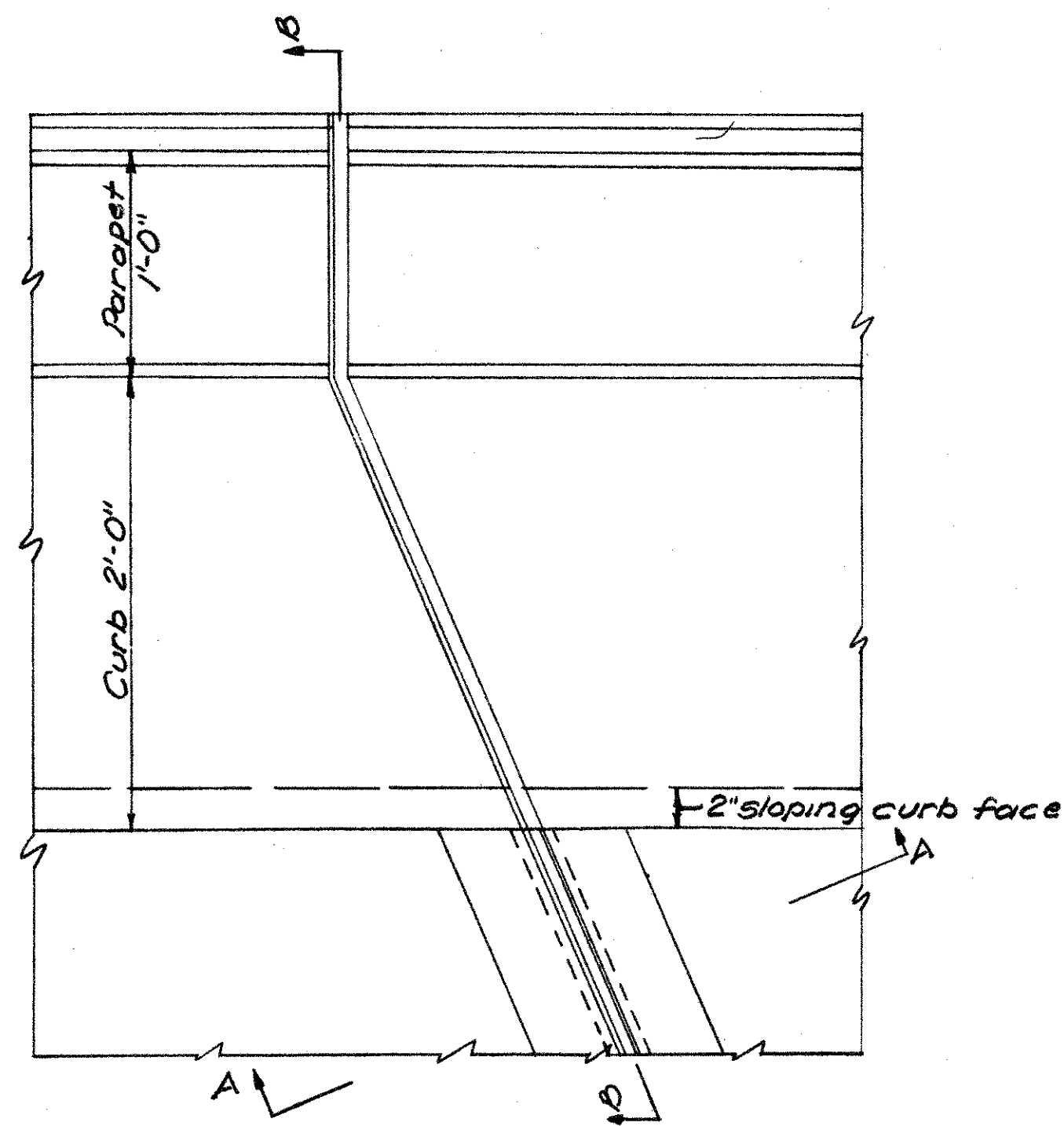
TYPICAL TRANSVERSE SECTION

- NOTES**
1. Slab thickness includes 1" monolithic wearing surface.
 2. Spread or cut longitudinal reinf steel in slab to clear scuppers.
 3. For end dam details see 6td. Dwg. SD-1-65, sheet 1.
 4. For aluminum railing details, see 6td. Dwg. BR-1-65, sheet 1.
 5. For curb plate details see 6td. Dwg. SD-1-65, sheet 2.
 6. For railing and parapet joint details see 6td. Dwg. BR-1-65, sheet 1.
 7. For scupper details see 6td. Dwg. SD-1-65, sheet 2.
 8. For pavement screed elevations see sheet 9.
 9. For light pole support detail see sheet 373.

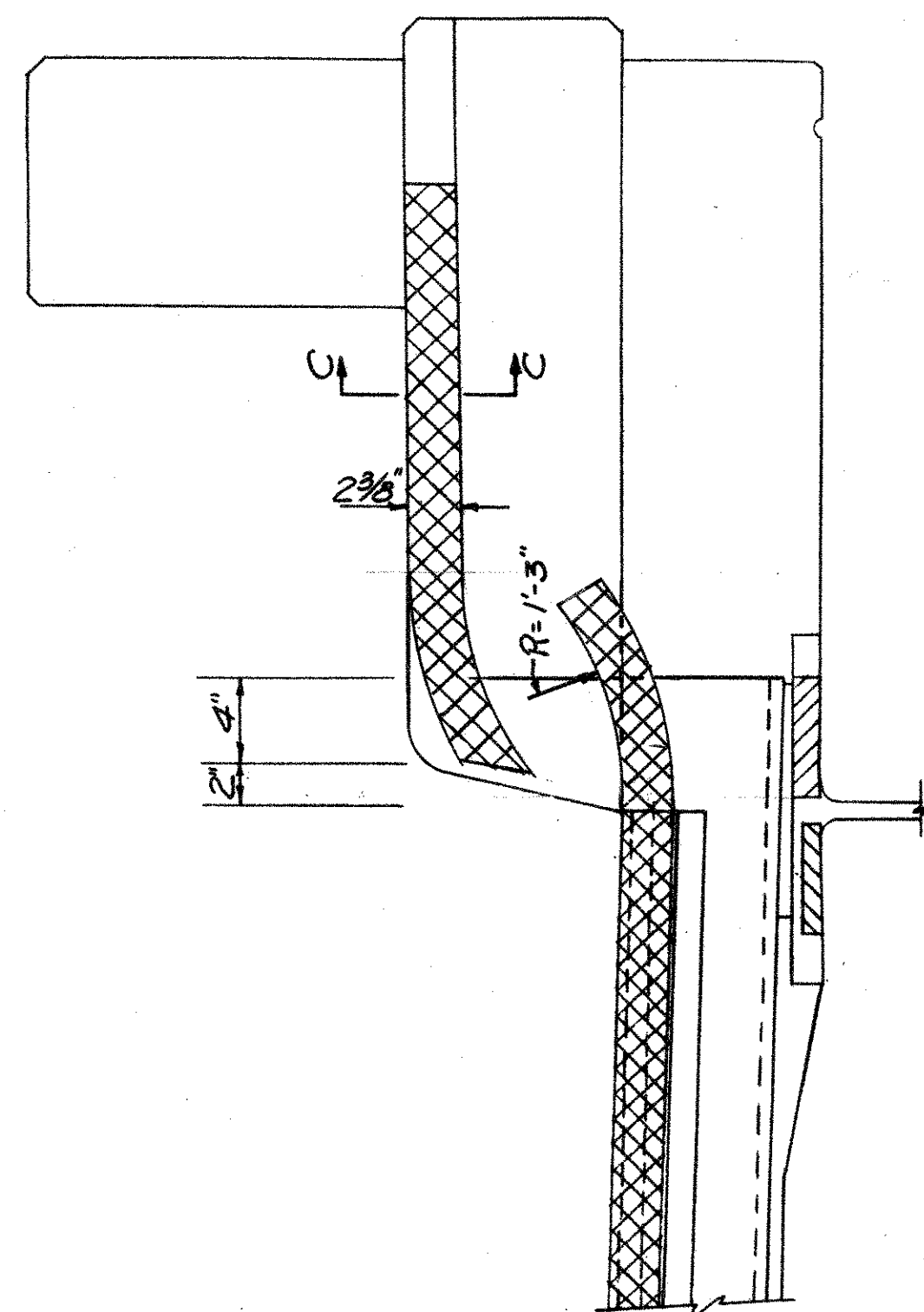
DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of the steel beams, which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" except that the maximum slope shall not exceed 3" per foot. Payment for deck slab concrete shall be based on the 9" width.

* This is a nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension even though deviation from it may be necessary because the top flanges of the beams may not have the exact camber or conformation required to place it parallel to the finished grade.

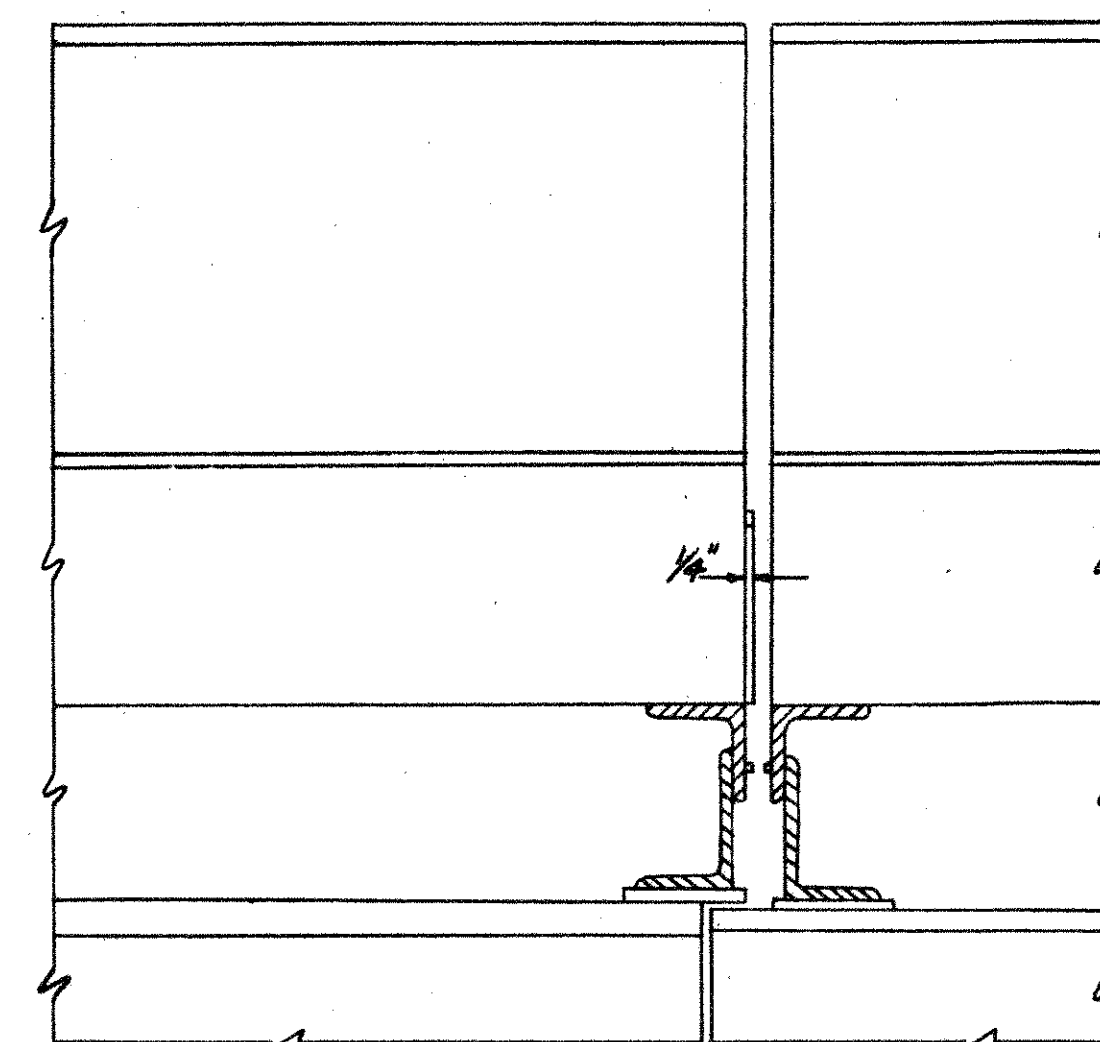
SHAW, LENZ & ASSOCIATES		8/10
CINCINNATI ENGINEERS		OHIO
SUPERSTRUCTURE SLAB		
BRIDGE NO. MDT-75-2127		
LITTLE YORK RD. OVER I-75		
I-75		
MONTGOMERY CO		STA. 263+44.24
DESIGNED	DRAWN	TRACED
W.B.S.	R.R.L.	R.J.L.
CHECKED	REVIEWED	REVISED



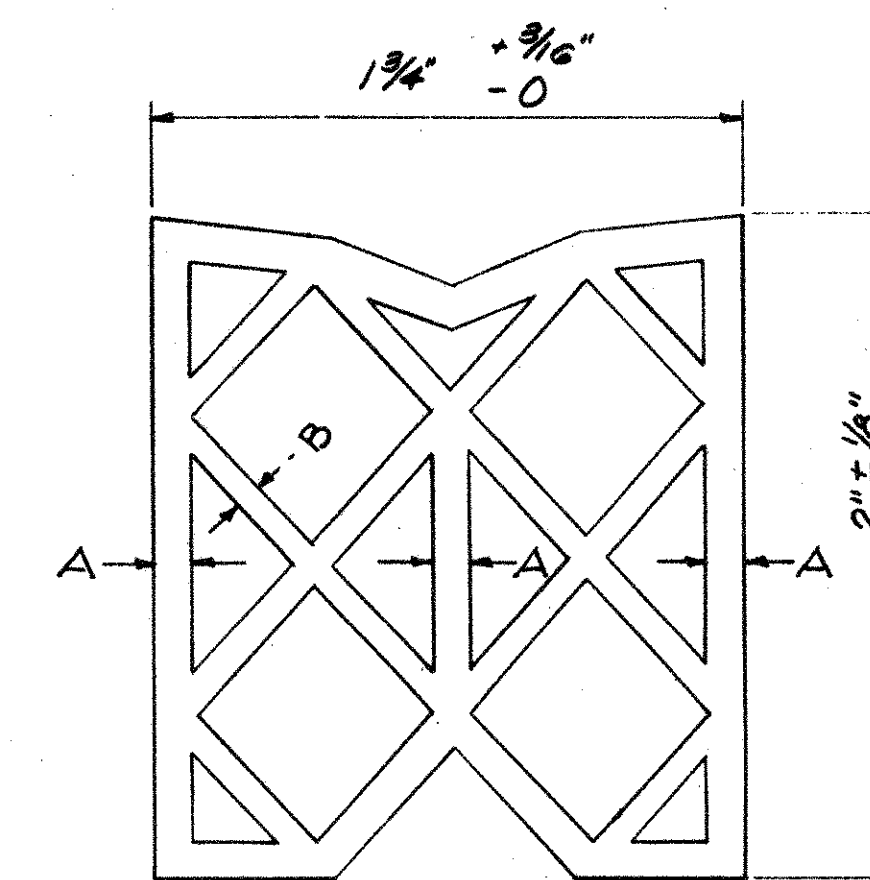
DETAIL "A"
PART DECK PLAN AT HINGE JOINT



SECTION "B-B"

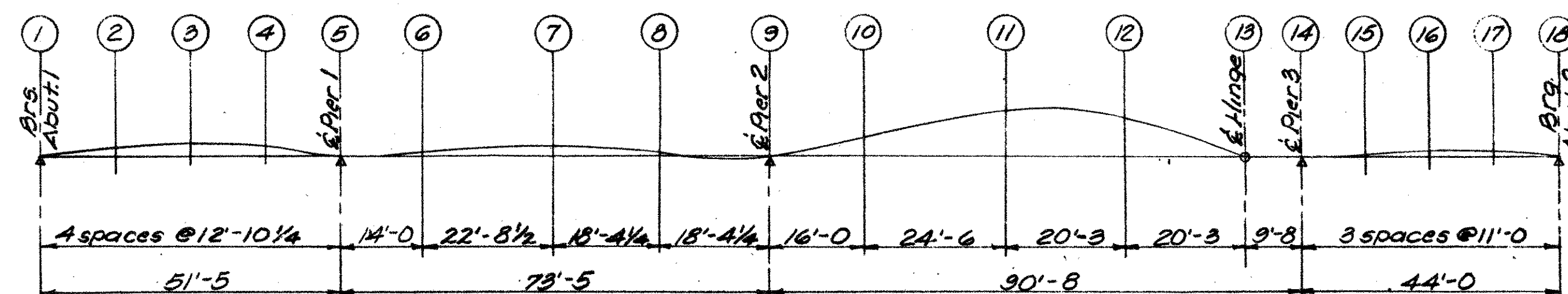


SECTION "A-A"



SECTION C-C
PREFORMED ELASTIC
JOINT SEALER

A = 1/8 + 1/32 - 1/64
B = 3/32 + 1/32 - 1/64



SCREED ELEVATION DIAGRAM

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
Station	18+74.30	+87.15	19+00.00	19+12.86	+25.71	+39.71	+62.42	+80.77	+93.13	20+15.13	+39.63	+59.88	+80.13	+89.80	21+00.80	+11.80	+22.80	21+33.80
Profile Grade	947.857	947.731	947.587	947.424	947.243	947.026	946.626	946.262	945.860	945.479	944.842	944.264	943.642	943.329	942.960	942.577	942.182	941.772
Gutter Elev. 15' Left	947.622	7.496	7.352	7.190	7.009	6.791	6.392	6.027	5.626	5.245	4.607	4.030	3.407	3.094	2.725	2.343	1.947	1.538
Concrete D.L. deflection	0.000	0.017	0.019	0.009	0.000	0.006	0.014	0.001	0.000	0.031	0.078	0.065	0.007	0.000	0.001	0.006	0.006	0.000
Screed Elev. Left Gutter	947.62	947.51	947.37	947.20	947.01	946.80	946.41	946.03	945.63	945.28	944.68	944.09	943.41	943.09	942.73	942.35	941.95	941.54
Station	18+87.12	+99.97	19+12.82	+25.68	+38.53	+52.53	+75.24	+93.59	20+11.95	+27.95	+52.45	+72.70	+92.95	21+02.62	+13.62	+24.62	+35.62	21+46.62
Profile Grade	947.731	947.587	947.425	947.244	947.045	946.807	946.376	945.985	945.557	945.154	944.481	943.875	943.224	942.897	942.513	942.115	941.703	941.279
Gutter Elev. 15' Left	947.497	7.352	7.190	7.009	6.810	6.573	6.141	5.751	5.323	4.920	4.247	3.641	2.990	2.663	2.278	1.880	1.469	1.044
Concrete D.L. deflection	0.000	0.017	0.019	0.009	0.000	0.006	0.014	0.001	0.000	0.034	0.078	0.065	0.007	0.000	0.000	0.006	0.006	0.000
Screed Elev. Right Gutter	947.50	947.37	947.21	947.02	946.81	946.58	946.16	945.75	945.32	944.95	944.32	943.71	943.00	942.66	942.28	941.89	941.47	941.04

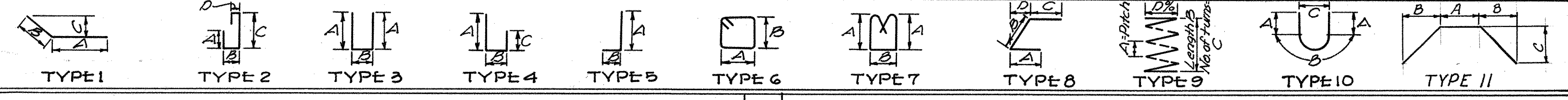
SHAW, LENZ & ASSOCIATES 9/10
ENGINEERS
CINCINNATI OHIO

SUPERSTRUCTURE SLAB
DETAILS

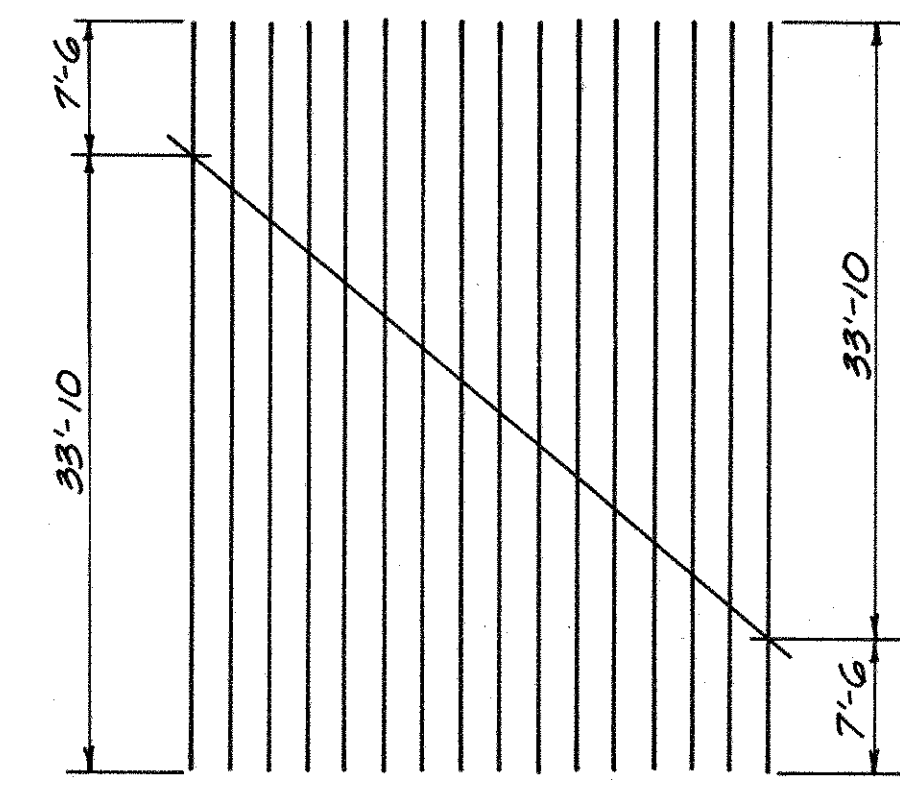
BRIDGE NO. MOT. 75-2127
LITTLE YORK OVER I-75

MONTGOMERY CO. STA. 268+44.24
DESIGNED DRAWN TRACED CHECKED REVIEWED REVISED
R.J.L. W.T.R.

REINFORCING STEEL BAR SCHEDULE



MARK	NO.	SIZE	LENGTH	TYPE	A	B	C	D	WEIGHT	LOCATION
ABUTMENTS										
A801	14		40'-8"	Str.					1,520	Ftg., Abuts. #1 & #2
A802	4		12'-7"	Str.					194	Ftg., Abut. #1
A803	5		11'-3"	Str.					150	Ftg., Abuts. #1 & #2
A804	5		12'-4"	Str.					165	Ftg., Abuts. #1 & #2
A805	2		12'-0"	1	8'-1"	3'-11"	0'-6"		64	Ftg., Abut. #1
A806	2		11'-7"	Str.					62	Ftg., Abut. #1 & #2
A807	1		12'-1"	Str.					32	Ftg., Abut. #2
A808	1		11'-0"	Str.					32	Ftg., Abut. #2
A809	4		11'-0"	Str.					117	Ftg., Abut. #2
SUPERSTRUCTURE										
A549	2		14'-2"	Str.					30	Curb, W.W. #4
A550	12		13'-5"	Str.					163	Curb, W.W. #3 & #4
R501	8		14'-4"	Str.					*	Parapets, Abut. #1
R502	8		13'-1"	Str.					*	Parapets, Abut. #2
R503	12								*	Parapets & endposts
R504	8								*	Parapets & endposts
PIERS										
P1101	40		7'-6"	5	5'-8"	2'-1"			1,534	Ftgs. 1 & 2
P1102	20		20'-7"	5					2,187	Cols. 1
P1103	20		19'-9"	5					2,099	Cols. 2
P1001	28		7'-0"	5	5'-4"	1'-11"			343	Ftg. 3
P1002	28		18'-8"	5					2,203	Cols. 3
P1003	6		34'-7"	5					893	Caps. 1, 2 & 3
P1004	6		31'-4"	5					809	Caps. 1, 2 & 3
P1005	4		11'-6"	10	4'-0"	3'-6"	2'-4"		193	Caps. 1 & 2
P1006	2		11'-2"	10	4'-0"	3'-2"	2'-1 1/2"		96	Cap 3
P1007	12		18'-0"	5					929	Caps. 1, 2 & 3
P1008	6		41'-0"	3	3'-3"	34'-11"			1,059	Caps. 1, 2 & 3
P1009	6		38'-6"	3	3'-3"	32'-5"			994	Caps. 1, 2 & 3
P901	51		30'-0"	5					5,202	Ftgs. 1, 2 & 3
P902	51		16'-0"	5					2,774	Ftgs. 1, 2 & 3
P903	54		23'-0"	5					4,223	Ftgs. 1, 2 & 3
P904	30		6'-4"	5	4'-11"	1'-7"			646	Ftgs. 1, 2 & 3
P905	10		19'-9"	5					672	Cols. 2
P906	10		20'-7"	5					700	Cols. 1
P907	10		18'-4"	5					623	Cols. 3
P908	10		34'-7"	5					1,176	Caps. 1, 2 & 3
P909	2		30'-6"	5					207	Cap 3
P801	168		6'-8"	5					1,682	Ftgs. 1, 2 & 3
P501	6		32'-1"	5					201	Caps. 1, 2 & 3
P502	72		7'-1"	3	2'-4"	2'-8"			532	Caps. 1, 2 & 3
SP401	3			9	4 1/2"	17'-6"	50	2'-8"	969	Cols. 1
SP402	3			9	4 1/2"	16'-8"	47	2'-8"	912	Cols. 2
SP403	3			9	4 1/2"	15'-2"	43	2'-8"	834	Cols. 3



CUTTING DIAGRAM
for 5702 thru 5717
& 5602 thru 5617

NOTES:

1. All dimensions are out to out of bar.
 2. Radius dimensions are to outside of bar.
 3. The length of bent bars is measured along centerline.
 4. Bar size is indicated in the bar mark. The first digit where three digits are used, and the first two digits where four are used indicates the bar size.
 5. Four steel channel, tee or angle spacers, weighing approximately 0.68 lbs. per lin. ft. of spacer shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities for spiral bars.
- The length shown in the steel list for the spiral bars is the distance from the top of the footing to the bottom of the pier cap. The "No. of Turns" shown is the length divided by the pitch, plus 3 turns (total number of closed coils) expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item 509. 1/2 closed coils shall be provided at the ends of each spiral unit.
- * These bars included with Item 517 for payment.

REPLACEMENT BARS			
MARK	NO.	LENGTH	TYPE
RE 1101	1	8'-7"	Str.
RE 1001	1	8'-3"	
RE 901	1	7'-10"	
RE 801	1	7'-6"	
RE 701	2	7'-3"	
RE 601	3	6'-11"	
RE 501	1	6'-7"	Str.

SHAW, LENZ & ASSOCIATES 10/10
ENGINEERS

CINCINNATI OHIO

REINFORCING STEEL DETAILS

BRIDGE NO. MOT. 75-2127
LITTLE YORK OVER I-75

MONTGOMERY CO. STA 268+44.24

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	REVISED
R.J.L.	W.T.R.				