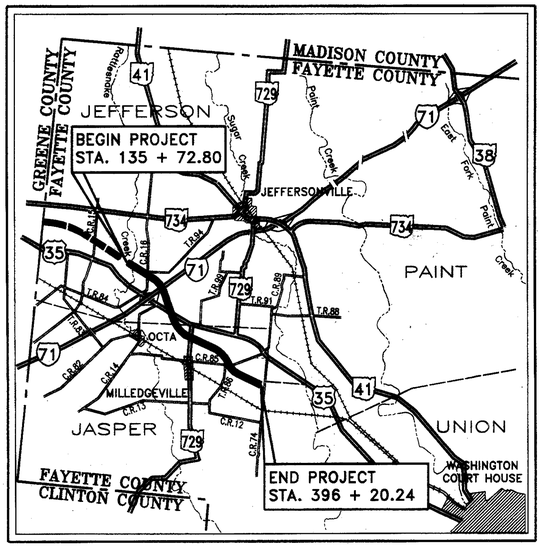
STATE OF OHIO
DEPARTMENT OF TRANSPORTATION
FAY-35-2.57
JEFFERSON TOWNSHIP
JASPER TOWNSHIP
FAYETTE COUNTY



LOCATION MAP

0 2 4
SCALE IN MILES

DESIGN DESIGNATION

SEE SCHEMATIC PLAN

DESIGN EXCEPTIONS

NONE REQUIRED

INDEX OF SHEETS

TITLE SHEET 1
SCHEMATIC LAYOUT & DESIGN DESIGNATION 2
REFERENCES 3–4
TYPICAL SECTIONS 5–11, 11A
GENERAL NOTES 12–13
MAINTENANCE OF TRAFFIC 14–32
GENERAL SUMMARY 33–36
SUB–SUMMARIES 37–42
CALCULATIONS 43–48, 48A, 48B
STORM WATER POLLUTION PREVENTION PLAN 49–53, 53A
PROPOSED U.S.R.35
PLAN & PROFILE 54–73
CROSS–SECTIONS 74–161
C.R.16
PLAN & PROFILE 162–163
CROSS–SECTIONS 164–173
T.R.94
PLAN & PROFILE 174
CROSS–SECTIONS 175–176
I–71
PLAN & PROFILE 177
CROSS–SECTIONS 178–181
EXISTING U.S.R.35
INTERCHANGE GEOMETRICS 182
PLAN & PROFILE 183–186
CROSS–SECTION LAYOUT 187
CROSS–SECTIONS 188–206
RAMP C
PLAN & PROFILE 207–208
CROSS–SECTIONS 209–215
RAMP D
PLAN & PROFILE 216
CROSS–SECTIONS 217–221
HARMONY ROAD
PLAN & PROFILE 221A
CROSS–SECTIONS 221B–221D
SERVICE ROADS NO.1 & NO.2
PLAN & PROFILE 222
CROSS–SECTIONS SERVICE RD. NO.1 223–225
CROSS–SECTIONS SERVICE RD. NO.2 226–227
PLAN & PROFILE SERVICE RD. DRIVES 228
CROSS–SECTIONS SERVICE RD. DRIVES 228A
SERVICE ROAD NO.3
PLAN & PROFILE 229–230
CROSS–SECTIONS 231–236
S.R.729
PLAN & PROFILE 237–239
CROSS–SECTIONS 240–254
C.R.85
CROSS–SECTIONS 255–258
MISCELLANEOUS DETAILS
APPROACH SLAB DETAILS 259–264
GRADING PLAN AT EX.U.S.R.35 265
GUARDRAIL DETAILS 266
PAVEMENT DETAILS 267–275
U–TURN MEDIAN OPENING DETAILS 276
SUPERELEVATION TABLES 277–278
DRAINAGE DETAILS
DRAINAGE SUB–SUMMARY 279
DRAINAGE INDEX 280–284
CULVERTS 285–312, 312A, 312B
STORM SEWERS 313–325
CHANNEL NO.1 326–328
CHANNEL NO.2 ..... 329–332A
UNDERDRAIN DETAILS 333–353
TRAFFIC CONTROL 354–383
LIGHTING ..... 384–392 B
STRUCTURES OVER 20' SPAN
FAY–35–0258 393–403
FAY–35–0308 404–416
FAY–35–0366 417–436
FAY–35–0435 437–454
FAY–35–0518 455–466
FAY–35–0644 467–482
RIGHT–OF–WAY 483–522

PROJECT DESCRIPTION

THIS PROJECT IS THE RELOCATION OF 4.93 MILES OF A DIVIDED RURAL EXPRESSWAY BETWEEN RATTLESNAKE CREEK AND HARMONY ROAD. THE IMPROVEMENT INCLUDES GRADE SEPARATIONS WITH COUNTY ROAD 16, INTERSTATE 71, EXISTING UNITED STATES ROUTE 35, STATE ROUTE 729, AND COUNTY ROAD 85. A PARTIAL INTERCHANGE IS INCLUDED WITH EXISTING UNITED STATES ROUTE 35.

LIMITED ACCESS

THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE REVISED CODE OF OHIO.

1997 SPECIFICATIONS

THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, INCLUDING CHANGES AND SUPPLEMENTAL SPECIFICATIONS LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY AND THAT PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE SET FORTH ON THE PLANS AND ESTIMATES.

PLANS CERTIFIED BY:
NAME: Dick Patrick DATE: 8/17/00
DISTRICT 6
OHIO DEPT. OF TRANSPORTATION

UNDERGROUND UTILITIES

TWO WORKING DAYS
BEFORE YOU DIG
CALL...1–800–362–2764 (TOLL FREE)
OHIO UTILITIES PROTECTION SERVICE
NON–MEMBERS
MUST BE CALLED DIRECTLY

PLANS PREPARED BY
ERIKSSON ENGINEERING, LTD.
1523 Chesapeake Ave.
Columbus, Ohio 43212
Tel. (614)488–0731

REVISED PAVED SHOULDER REVISIONS AS NOTED ON SHEET 12A.

STATE OF OHIO JULIE A. DONOVAN E-56075 REGISTERED PROFESSIONAL ENGINEER
SIGNED: Julie A. Donovan
DATE: 7/14/00

ENGINEERS SEAL

STATE OF OHIO CARL E. ERIKSSON 25802 REGISTERED PROFESSIONAL ENGINEER
SIGNED: Carl E Eriksson
DATE: May 18th 1999

STANDARD DRAWINGS

		GR–4.3M	10–21–97	HL–30.21M	5–1–95	TC–42.10M 3–31–94	AS–1–81 9-15-94
BP–3.1	7-28-00	GR–4.4M	11–30–94			TC–42.20M 3–31–94	BR–1 1-06-99
BP–4.1	7-28-00	GR–5.1M	4–21–95	HL–40.10M	3–31–95		BR–2 –98 12-29-98
BP–5.1	7-28-00	GR–5.2M	11–30–94	HL–50.11M	3–31–95	TC–51.11M 9–30–94	GSD– 1 – 96 2-12-96
		GR–5.3M	11–30–94	HL–60.11M	5–1–95		
CB–1.1M	7-12-95			HL–60.12M	3–31–95	TC–52.10M 7–29–94	
		GR–6.2M	1–3–96	HL–60.21M	3–31–95	TC–52.20M 7–29–94	ICD–1–82M 3-20-95
CB–3.1M	7-12-95			HL–60.31M	3–31–95	TC–61.10M 3–31–94	
CB–3.3M	7-12-95	HW–2.1M	7–12–95			TC–65.10M 11–1–95	CPP–2–94 12-19-94
CB–3.4M	7-12-95	HW–2.2M	7–12–95			TC–65.11M 11–1–95	
						TC–65.12M 11–1–95	CS–1–93 6-30-95
		RM–1.1	4-29-99			TC–72.20M 9–1–93	
F–2.1	7-28-00	DM–1.1M	10-21-97	TC–12.30M	2–1–94		
F–3.1	7-28-00	DM–4.1M	6–30–95				
		DM–4.2M	6–30–95			MT–35.10M 1–30–95	
F–3.3	7-28-00	DM–4.3	4-29-99	TC–21.20M	12–10–96		"SPECIAL PROVISIONS"
F–3.4	7-28-00	DM–4.4	4-29-99	TC–22.10M	3-13-97		
				TC–22.20M	2–1–94	MT–95.40M 4–25–94	
GR–1.1M	10–21–97	LA–1.1	7-28-00	TC–31.21M	3–31–94	MT–95.41M 4–25–94	
GR–1.2M	1–3–96			TC–32.10M	3–31–94	MT–97.10M 4–25–94	
		HL–10.31M	3–31–95			MT–97.11M 1–30–95	
GR–2.1M	4-14-98	HL–20.11M	3–31–95				
GR–3.1M	10–21–97	HL–20.21M	8–31–94	TC–41.10M	3–31–94	MT–101.60M 4–25–94	
GR–3.2M	10–21–97	HL–20.22M	3–31–95	TC–41.20M	7–1–94	MT–102.10M 1–30–95	
		HL–20.23M	3–31–95	TC–41.40M	3–31–94	MT–105.10M 4–25–94	
GR–4.2M	10–21–97	HL–30.11M	3–31–95	TC–41.50M	7–1–94	MT–105.11M 4–25–94	

SHEETS ADDED.....12A,228A,392A,392B

SHEETS DELETED.....314,323,324,325

SUPPLEMENTAL SPECIFICATIONS

806	9–9–97
863	10-12-99
880	6-15-99
814	6-2-98
830	10-21-98
877	4-13-99
899	10-21-98
887	8-10-99
905	4-1-98
906	5-5-98
907	10-21-98
908	3-28-00
870	8-10-99
816	4-21-97
828	7-28-98
842	1-06-99
844	1-06-99

Jeffery A. Crace
P.E. Reg. No.
E-53789
SIGNED: Jeffery A Crace
DATE: 7-31-00

APPROVED: Jack R. Marshall Jr.
DATE 8/17/00 DISTRICT DEPUTY DIRECTOR OF TRANSPORTATION

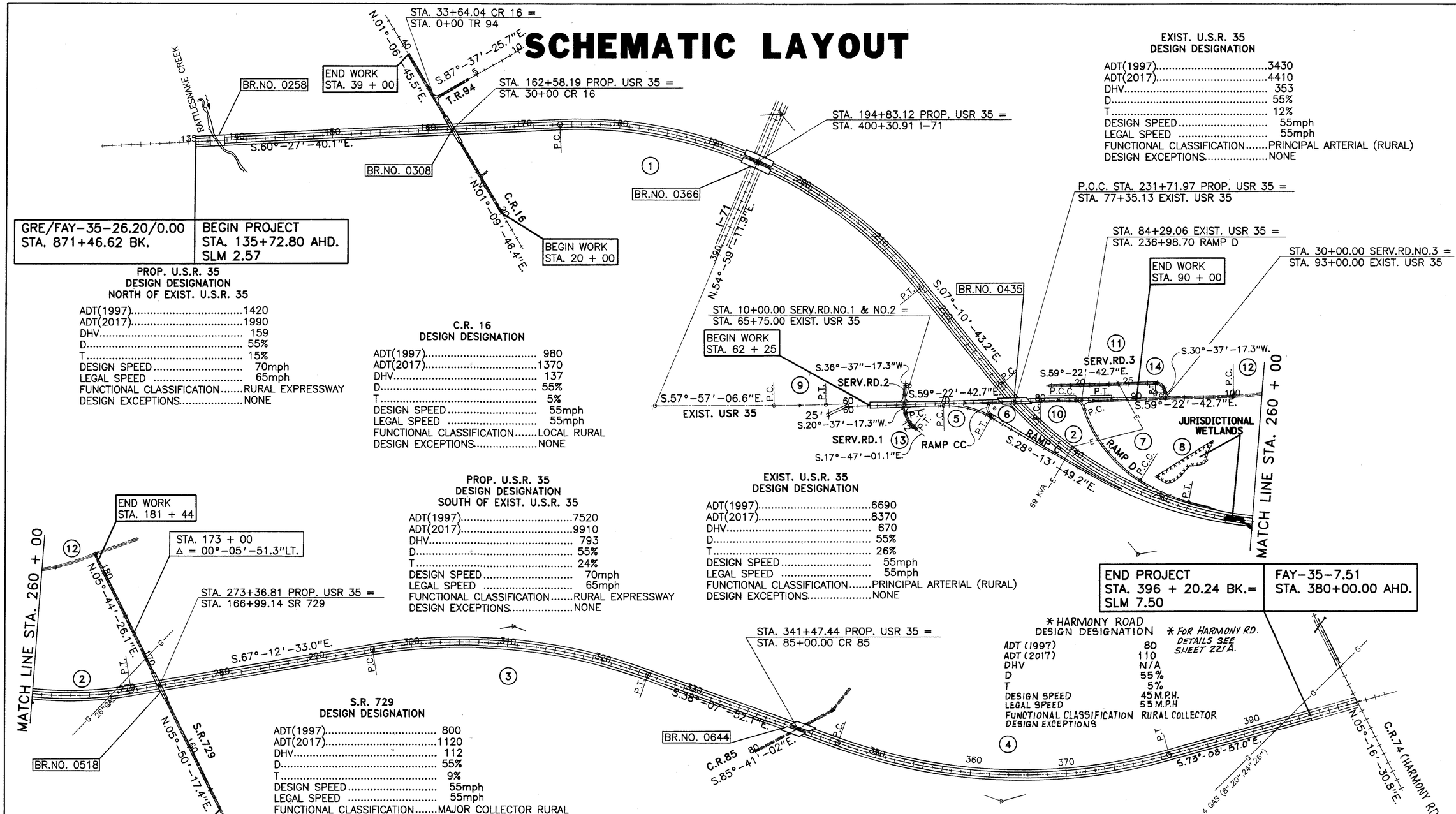
APPROVED: Gordon Proctor, P.E.
DATE 8-31-00 DIRECTOR, DEPARTMENT OF TRANSPORTATION

Revised 8/08/00 By: E.R.P.

FEDERAL PROJECT NO.
PID NO. 9078
CONSTRUCTION PROJECT NO.
RAILROAD INVOLVEMENT NONE
FAY–35–2.57
1 / 522

FAY - USR 35 - 2.57
000577
DIST 06
PID# 9078
12-13-00

SCHEMATIC LAYOUT



PROP. U.S.R. 35
DESIGN DESIGNATION
NORTH OF EXIST. U.S.R. 35

ADT(1997)	1420
ADT(2017)	1990
DHV	159
D	55%
T	15%
DESIGN SPEED	70mph
LEGAL SPEED	65mph
FUNCTIONAL CLASSIFICATION	RURAL EXPRESSWAY
DESIGN EXCEPTIONS	NONE

C.R. 16
DESIGN DESIGNATION

ADT(1997)	980
ADT(2017)	1370
DHV	137
D	55%
T	5%
DESIGN SPEED	55mph
LEGAL SPEED	55mph
FUNCTIONAL CLASSIFICATION	LOCAL RURAL
DESIGN EXCEPTIONS	NONE

PROP. U.S.R. 35
DESIGN DESIGNATION
SOUTH OF EXIST. U.S.R. 35

ADT(1997)	7520
ADT(2017)	9910
DHV	793
D	55%
T	24%
DESIGN SPEED	70mph
LEGAL SPEED	65mph
FUNCTIONAL CLASSIFICATION	RURAL EXPRESSWAY
DESIGN EXCEPTIONS	NONE

EXIST. U.S.R. 35
DESIGN DESIGNATION

ADT(1997)	6690
ADT(2017)	8370
DHV	670
D	55%
T	26%
DESIGN SPEED	55mph
LEGAL SPEED	55mph
FUNCTIONAL CLASSIFICATION	PRINCIPAL ARTERIAL (RURAL)
DESIGN EXCEPTIONS	NONE

EXIST. U.S.R. 35
DESIGN DESIGNATION

ADT(1997)	3430
ADT(2017)	4410
DHV	353
D	55%
T	12%
DESIGN SPEED	55mph
LEGAL SPEED	55mph
FUNCTIONAL CLASSIFICATION	PRINCIPAL ARTERIAL (RURAL)
DESIGN EXCEPTIONS	NONE

S.R. 729
DESIGN DESIGNATION

ADT(1997)	800
ADT(2017)	1120
DHV	112
D	55%
T	9%
DESIGN SPEED	55mph
LEGAL SPEED	55mph
FUNCTIONAL CLASSIFICATION	MAJOR COLLECTOR RURAL
DESIGN EXCEPTIONS	NONE

*** HARMONY ROAD**
DESIGN DESIGNATION
* FOR HARMONY RD. DETAILS SEE SHEET 22/A.

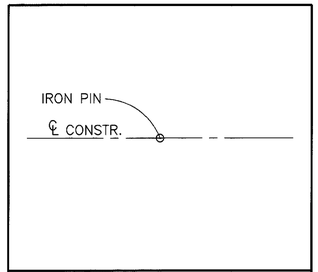
ADT (1997)	80
ADT (2017)	110
DHV	N/A
D	55%
T	5%
DESIGN SPEED	45 M.P.H.
LEGAL SPEED	55 M.P.H.
FUNCTIONAL CLASSIFICATION	RURAL COLLECTOR
DESIGN EXCEPTIONS	

	PROPOSED USR 35				RAMP C	RAMP CC	RAMP D		EXISTING USR 35				SERVICE ROADS NO.1	SERVICE ROADS NO.3
CURVE NUMBER	1	2	3	4	5	6	7	8	9	10	11	12	13	14
P.I. STATION	196 +74.95	252 +01.07	310 +76.19	363 +90.54	228 +06.35	229 +94.24	242 +91.95	250 +25.81	55 +01.83	81 +85.44	85 +56.28	104 +54.03	11 +73.14	28 +80.92
P.C. STATION	173 +75.49	229 +44.25	295 +90.26	345 +83.02	225 +40.19	229 +01.04	238 +00.00	PCC 247 +47.74	52 +34.31	80 +00.00	PRC 83 +70.84	100 +11.85	10 +86.07	27 +80.92
P.T. STATION	216 +38.09	270 +37.24	324 +98.06	380 +84.82	230 +59.33	230 +41.01	PCC 247 +47.74	253 +00.00	57 +69.33	PRC 83 +70.84	87 +41.68	108 +89.29	12 +53.64	29 +38.00
Δ	53°-16'-56.9"RT.	60°-01'-49.8"LT.	29°-04'-40.9"RT.	35°-01'-04.9"LT.	31°-08'-53.5"RT.	96°-37'-33.2"LT.	37°-54'-35.2"LT.	16°-34'-03.6"LT.	1°-25'-36.2"LT.	1°-51'-15.1"RT.	1°-51'-15.1"LT.	17°-32'-52.4"LT.	38°-24'-18.4"LT.	90°-00'-00"RT.
Dc	1°-15'	1°-28'	1°-00'	1°-00'	6°-00'	69°-01'-51.8"	4°-00'	3°-00'	0°-16'	0°-30'	0°-30'	2°-00'	22°-55'-06"	57°-17'-45"
R	4583.66'	3906.53'	5729.58'	5729.58'	954.93'	83.00'	1432.39'	1909.86'	21,485.93'	11,459.16'	11,459.16'	2864.93'	250.00'	100.00'
T	2299.46'	2256.82'	1485.93'	1807.52'	266.16'	93.20'	491.95'	278.07'	267.52'	185.44'	185.44'	442.18'	87.07'	100.00'
L	4262.60'	4092.99'	2907.80'	3501.80'	519.14'	139.97'	947.74'	552.26'	535.02'	370.84'	370.84'	877.44'	167.57'	157.08'
E	544.44'	605.04'	189.55'	278.35'	36.40'	41.80'	82.13'	20.14'		6.00'	6.00'	33.92'	14.73'	41.42'
SUPERELEVATION	.044 : 1	.051 : 1	.036 : 1	.036 : 1	.044 : 1		.051 : 1	.051 : 1					.022:1	.016:1

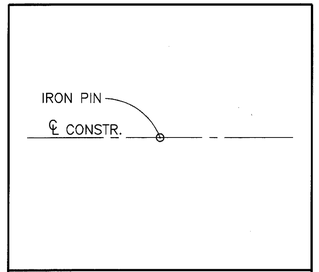
REVISED 1/00 BY FYC
CALCULATED
CHECKED

SCHEMATIC LAYOUT

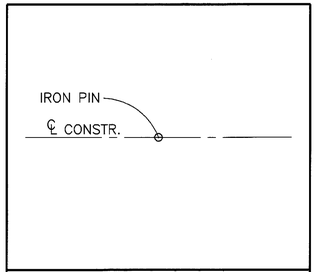
FAY-35-2.57



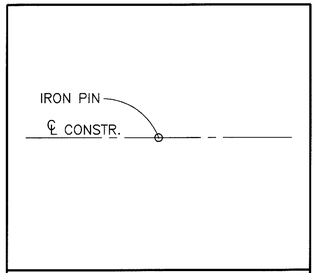
POT STA. 136+70 PROP.USR 35



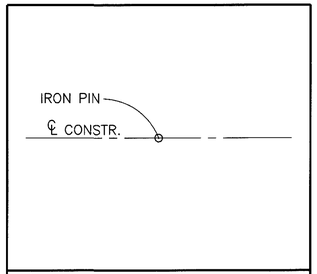
POT STA. 139+45 PROP.USR 35



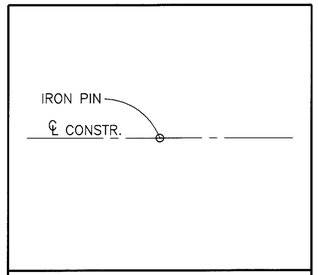
POT STA. 154+80 PROP.USR 35



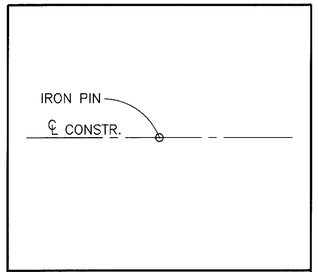
PC STA. 173+75.49 PROP.USR 35



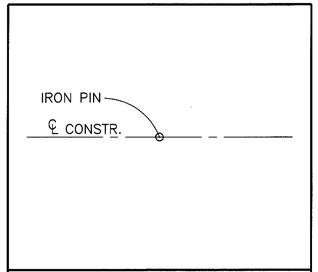
POC STA. 193+00 PROP.USR 35



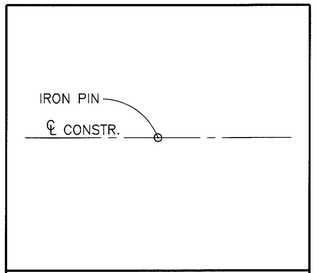
POC STA. 196+00 PROP.USR 35



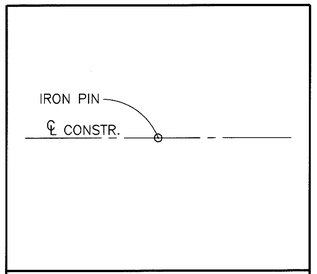
PT STA. 216+38.09 PROP.USR 35



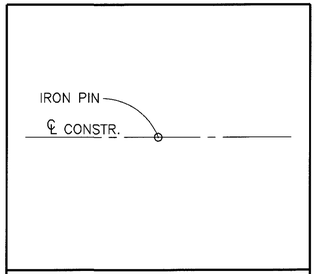
PC STA. 229+44.25 PROP.USR 35



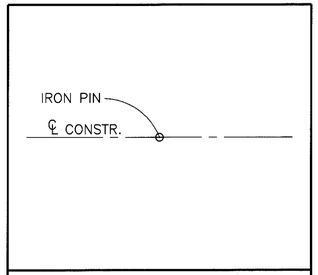
POC STA. 240+00 PROP.USR 35



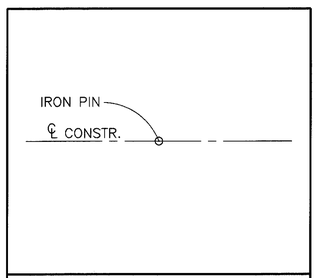
POC STA. 251+25 PROP.USR 35



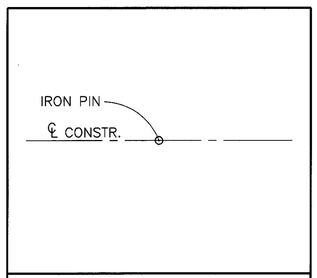
POC STA. 262+00 PROP.USR 35



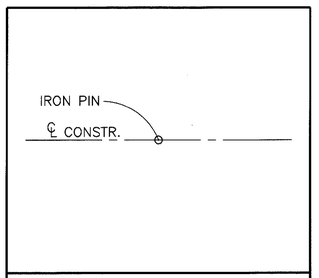
PT STA. 270+37.24 PROP.USR 35



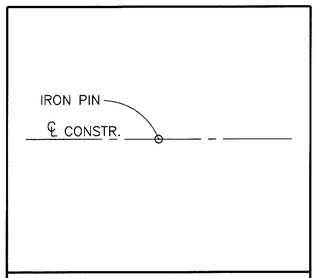
POT STA. 285+80 PROP.USR 35



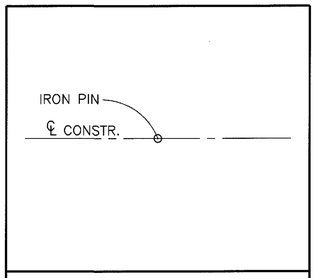
POT STA. 289+00 PROP.USR 35



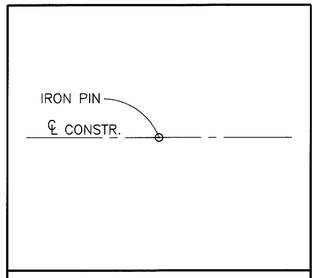
PC STA. 295+90.26 PROP.USR 35



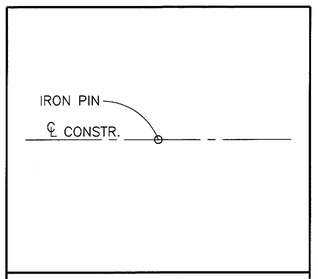
POC STA. 311+60 PROP.USR 35



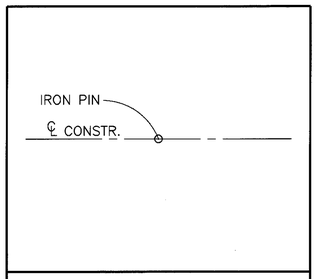
PT STA. 324+98.06 PROP.USR 35



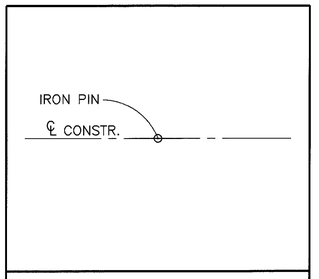
PC STA. 345+83.02 PROP.USR 35



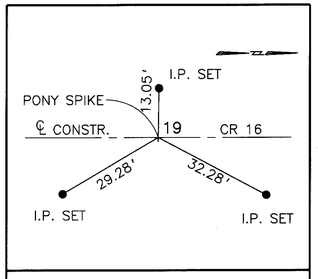
POC STA. 370+00 PROP.USR 35



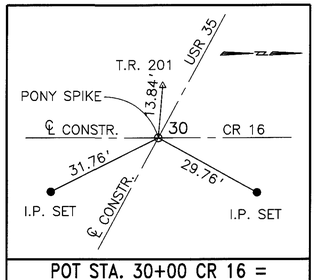
PT STA. 380+84.82 PROP.USR 35



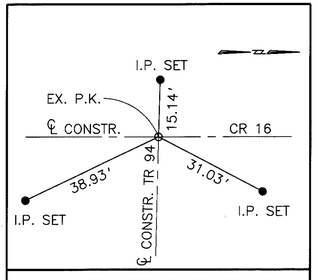
POT STA. 386+75 PROP.USR 35



POT STA. 19+00 CR 16



POT STA. 30+00 CR 16 =
POT STA. 162+58.19 PROP.USR 35

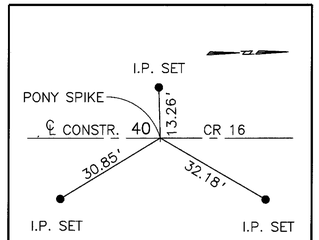


POT STA. 33+64.22 CR 16

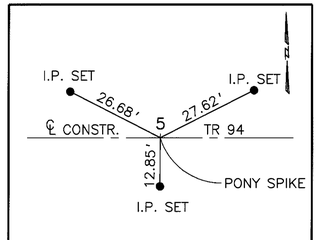
CALCULATED | CHECKED

REFERENCES

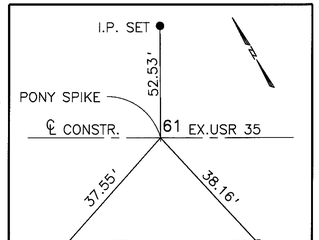
FAY-35-2.57



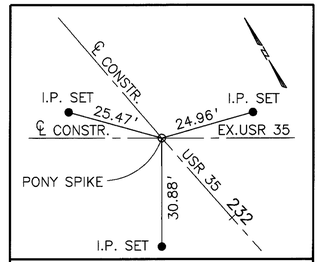
POT STA. 40+00 CR 16



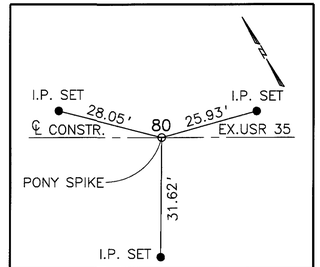
POT STA. 5+00 TR 94



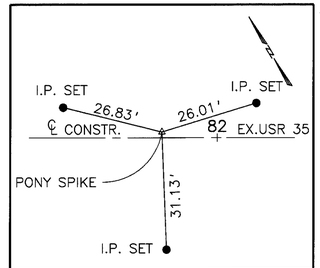
POT STA. 61+00 EX.USR 35



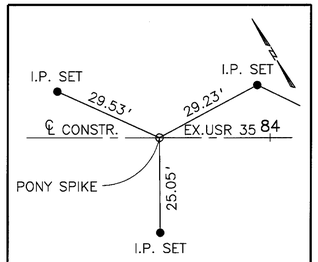
POT STA. 77+35.12 EX.USR 35 =
POC STA. 231+71.97 PROP.USR 35



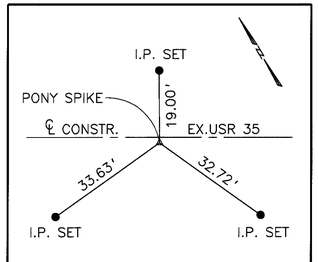
PC STA. 80+00 EX.USR 35



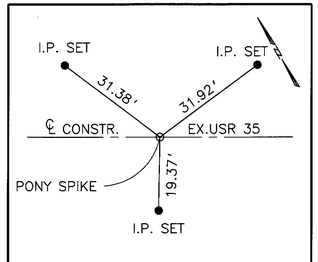
PI STA. 81+85.44 EX.USR 35



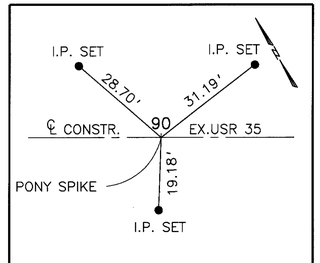
PRC STA. 83+70.84 EX.USR 35



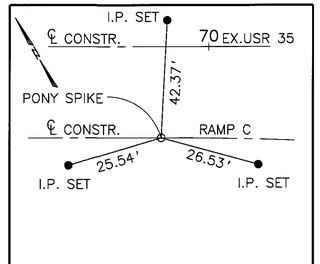
PI STA. 85+56.28 EX.USR 35



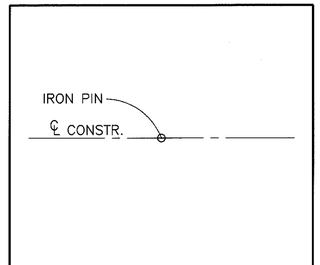
PT STA. 87+41.68 EX.USR 35



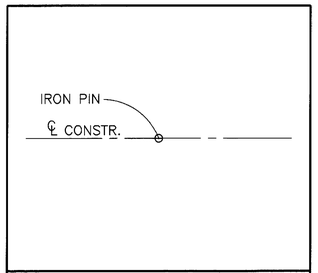
POT STA. 90+00 EX.USR 35



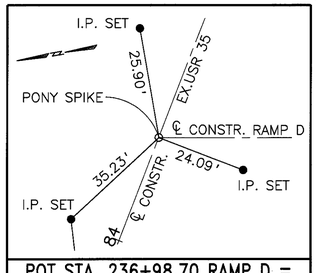
PC STA. 225+40.19 RAMP C



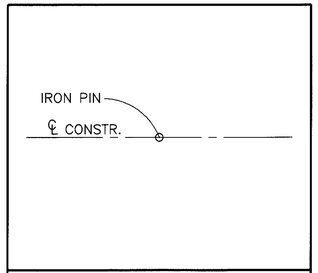
PT STA. 230+59.33 RAMP C



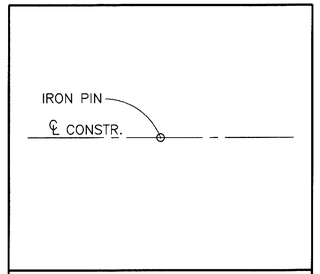
POT STA. 239+00 RAMP C



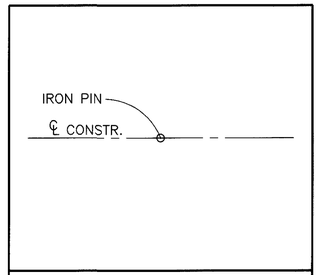
POT STA. 236+98.70 RAMP D =
POC STA. 84+29.06 EX.USR 35



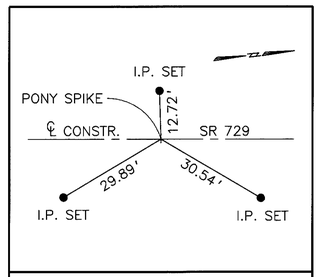
PC STA. 238+00 RAMP D



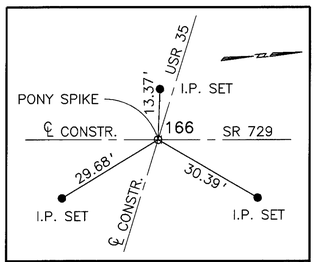
PRC STA. 247+47.74 RAMP D



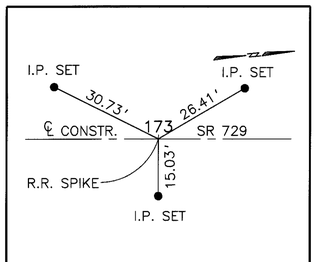
PT STA. 253+00 RAMP D



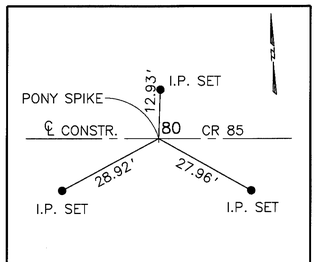
POT STA. 150+00 SR 729



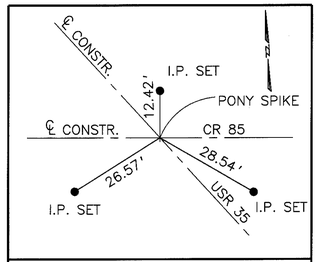
POT STA. 166+99.14 SR 729 =
POT STA. 273+36.81 PROP.USR 35



POT STA. 173+00 SR 729



POT STA. 80+00 CR 85



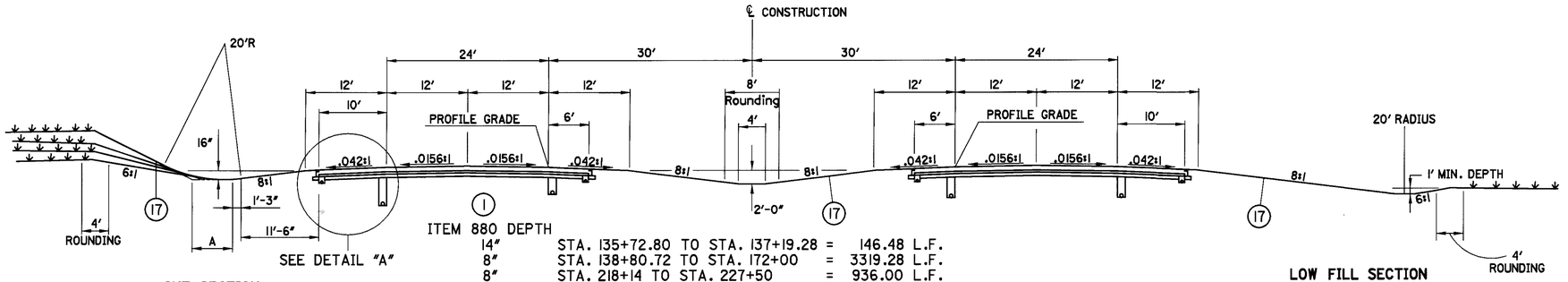
POT STA. 85+00 CR 85 =
POT STA. 341+47.44 PROP.USR 35

CALCULATED | CHECKED

REFERENCES

FAY-35-2.57

* SUPERELEVATION SLOPE, 0.042:1 OR PAVEMENT SLOPE WHICHEVER IS GREATER.



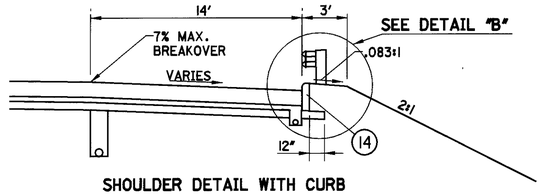
DITCH WIDTHS (A)
BACKSLOPE 6:1 = 3'-0"
BACKSLOPE 4:1 = 3'-9"
BACKSLOPE 3:1 = 4'-6"
BACKSLOPE 2:1 = 6'-0"

CUT SECTION

LOW FILL SECTION

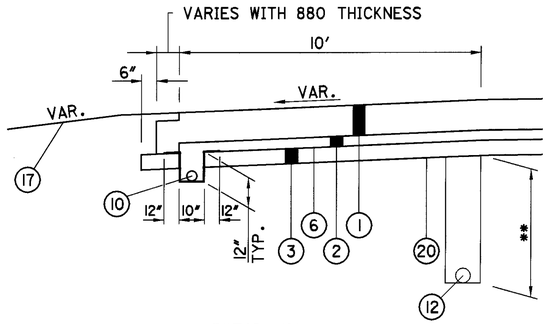
ITEM 880 DEPTH		
14"	STA. 135+72.80 TO STA. 137+19.28 =	146.48 L.F.
8"	STA. 138+80.72 TO STA. 172+00 =	3319.28 L.F.
8"	STA. 218+14 TO STA. 227+50 =	936.00 L.F.
12"	STA. 272+31 TO STA. 294+50 =	2219.00 L.F.
12"	STA. 326+41 TO STA. 340+44.92 =	1403.92 L.F.
12"	STA. 342+49.96 TO STA. 344+50 =	200.04 L.F.
12"	STA. 382+25 TO STA. 396+20.24 =	1295.24 L.F.
	TOTAL =	9519.96 L.F.

NORMAL SECTION



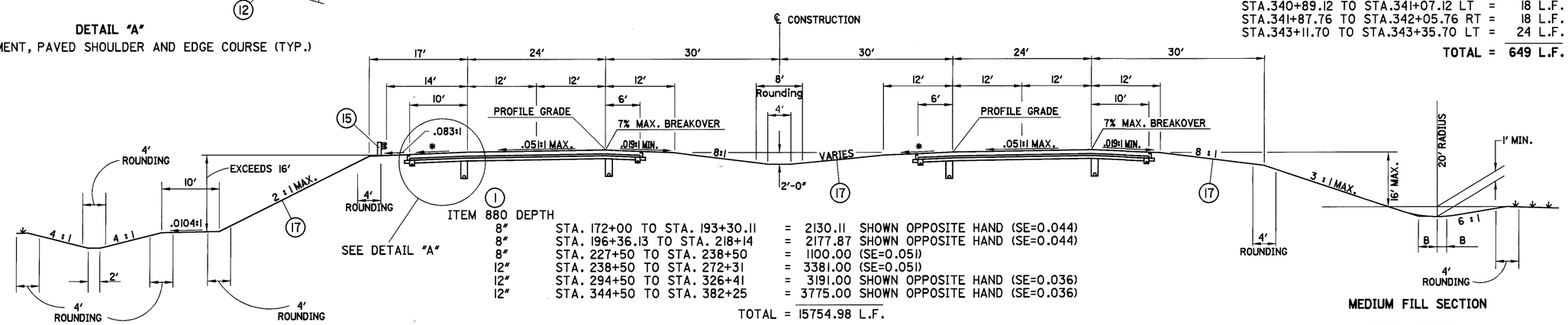
SHOULDER DETAIL WITH CURB

STA.136+60.31 TO STA.136+80.31 LT	=	20 L.F.
STA.137+34.25 TO STA.137+58.25 RT	=	24 L.F.
STA.193+04.76 TO STA.193+29.13 RT	=	24 L.F.
STA.193+06.80 TO STA.193+30.44 LT	=	24 L.F.
STA.196+34.25 TO STA.196+57.80 LT	=	24 L.F.
STA.196+38.17 TO STA.200+93.98 RT	=	449 L.F.
STA.339+59.18 TO STA.339+83.18 RT	=	24 L.F.
STA.340+89.12 TO STA.341+07.12 LT	=	18 L.F.
STA.341+87.76 TO STA.342+05.76 RT	=	18 L.F.
STA.343+11.70 TO STA.343+35.70 LT	=	24 L.F.
TOTAL	=	649 L.F.



DETAIL "A"
PAVEMENT, PAVED SHOULDER AND EDGE COURSE (TYP.)

** 30" SHALLOW PIPE UNDERDRAIN
50" DEEP PIPE UNDERDRAIN



ITEM 880 DEPTH			
8"	STA. 172+00 TO STA. 193+30.11	=	2130.11 SHOWN OPPOSITE HAND (SE=0.044)
8"	STA. 196+36.13 TO STA. 218+14	=	2177.87 SHOWN OPPOSITE HAND (SE=0.044)
8"	STA. 227+50 TO STA. 238+50	=	1100.00 (SE=0.051)
12"	STA. 238+50 TO STA. 272+31	=	3381.00 (SE=0.051)
12"	STA. 294+50 TO STA. 326+41	=	3191.00 SHOWN OPPOSITE HAND (SE=0.036)
12"	STA. 344+50 TO STA. 382+25	=	3775.00 SHOWN OPPOSITE HAND (SE=0.036)
	TOTAL =		15754.98 L.F.

DEEP FILL SECTION

SUPERELEVATED SECTION

MEDIUM FILL SECTION

DITCH WIDTHS (B)
BACKSLOPE 8:1 = 1'-3"
BACKSLOPE 6:1 = 1'-8"
BACKSLOPE 4:1 = 2'-6"
BACKSLOPE 3:1 = 3'-3"

LEGEND

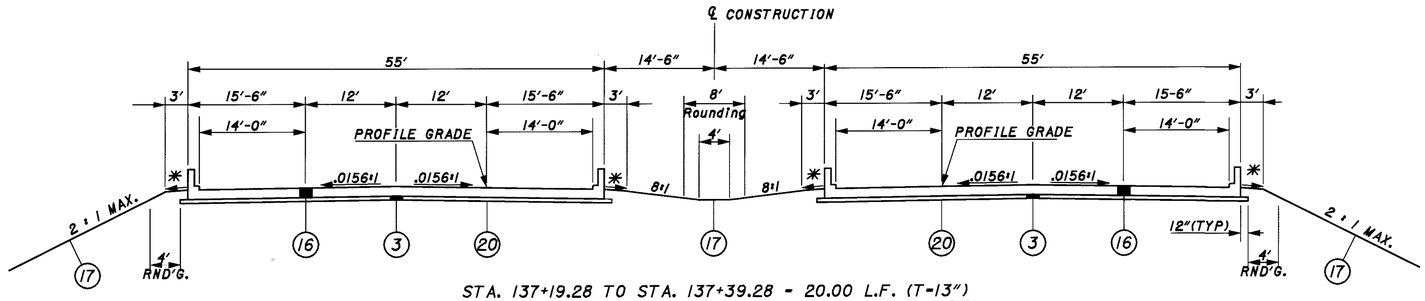
(1)	ITEM 880	ASPHALT CONCRETE (7 YEAR WARRANTY) (depth varies)	(8)	ITEM 304	8" AGGREGATE BASE
(2)	ITEM 307	4" NON-STABILIZED DRAINAGE BASE, TYPE IA	(9)	ITEM 304	AGGREGATE BASE (THICKNESS AS SHOWN)
(3)	ITEM 304	6" AGGREGATE BASE	(10)	ITEM 605	4" PIPE UNDERDRAIN, WITH FABRIC WRAP, AS PER PLAN (SEE SHEET 6)
(4)	ITEM 448	1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22	(12)	ITEM 605	6" PIPE UNDERDRAIN
(5)	ITEM 301	4" BITUMINOUS AGGREGATE BASE, PG64-22	(14)	ITEM 830	CURB, TYPE 6
(6)	ITEM 408	BITUMINOUS PRIME COAT, APPLIED AT THE RATE OF 0.4 GAL./S.Y.	(15)	ITEM 606	GUARDRAIL, TYPE 5
(7)	ITEM 301	8" BITUMINOUS AGGREGATE BASE, PG64-22	(16)	ITEM 611	REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (T=AS SHOWN) (SEE SHEET 13)
			(17)	ITEM 659	SEEDING AND MULCHING
			(19)	ITEM 605	4" PIPE UNDERDRAIN
			(20)	ITEM 203	SUBGRADE COMPACTION
			(21)	ITEM 605	AGGREGATE DRAIN
			(22)	ITEM 446	1¼" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
			(23)	ITEM 446	1¾" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
			(24)	ITEM 301	BITUMINOUS AGGREGATE BASE, PG64-22 (DEPTH AS SHOWN)
			(25)	ITEM 407	TACK COAT FOR INTERMEDIATE COURSE (SEE GENERAL NOTE)

CALCULATED J.L.G. | CHECKED W.G.Y.
REVISED 1/00 | BY FYC

TYPICAL SECTIONS - PROPOSED U.S.R. 35

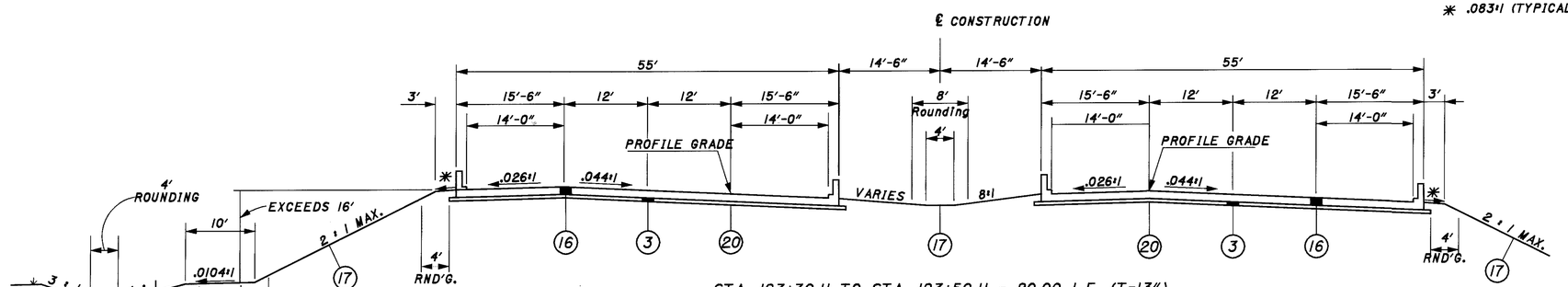
FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\fay35gy1.dgn



STA. 137+19.28 TO STA. 137+39.28 - 20.00 L.F. (T=13")
STA. 138+60.72 TO STA. 138+80.72 - 20.00 L.F. (T=13")
STA. 340+44.92 TO STA. 340+69.92 - 25.00 L.F. (T=15")
STA. 342+24.96 TO STA. 342+49.96 - 25.00 L.F. (T=15")

NORMAL SECTION

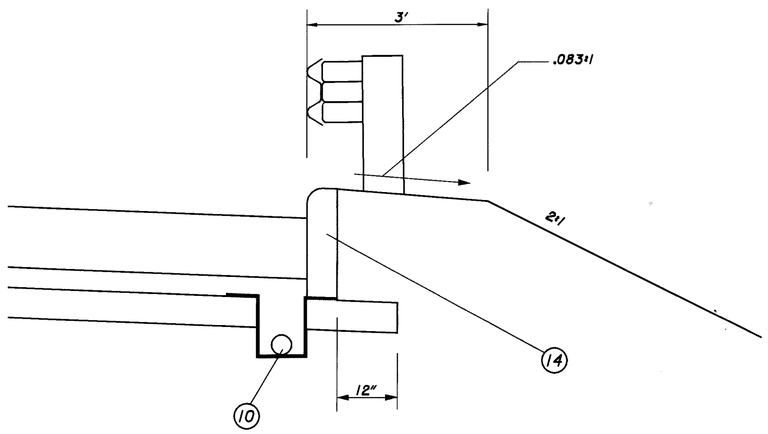


STA. 193+30.11 TO STA. 193+50.11 - 20.00 L.F. (T=13")
STA. 196+16.13 TO STA. 196+36.13 - 20.00 L.F. (T=13")

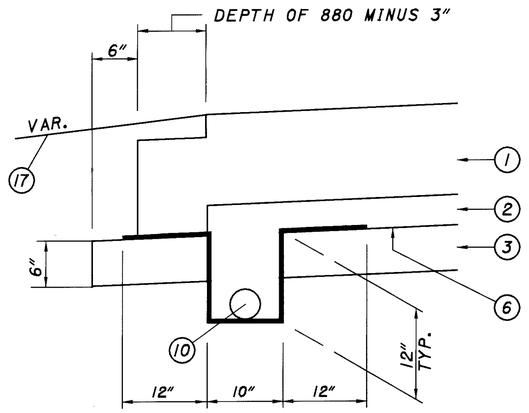
SUPERELEVATED SECTION

NOTE: SHOULDER CROSS SLOPES SHALL BE TRANSITIONED TO THE PAVEMENT SLOPES WITHIN THE APPROACH SLAB LENGTH ON EACH SIDE OF THE STRUCTURE.

TYPICAL DEEP FILL SECTION



CURB AND UNDERDRAIN DETAIL B



4" PIPE UNDERDRAIN, WITH FABRIC WRAP AS PER PLAN

NOTES:

1. DO NOT PLACE ITEM 408 WITHIN 1' (ONE FOOT) OF THE UNDERDRAIN. NO ITEM 408 OUTSIDE OF THE UNDERDRAIN.
2. MAXIMUM TRENCH WIDTH = 10".
3. THE MAXIMUM LIFT THICKNESS SHALL BE WAIVED FOR ITEM 880 COURSE AT THE OUTSIDE OF THE FREE DRAINING BASE.
4. THE FABRIC FILTER MATERIAL, 712.09, TYPE A SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEM 605 - 4" PIPE UNDERDRAIN, WITH FABRIC WRAP, AS PER PLAN.

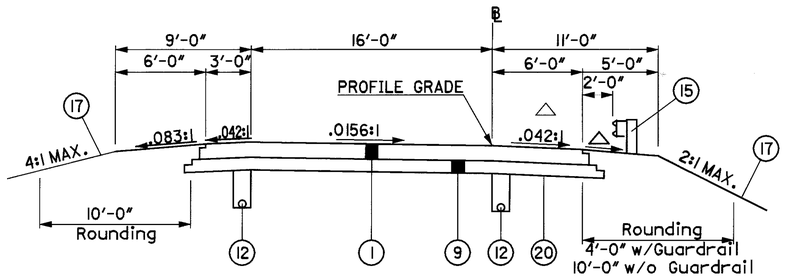
FOR LEGEND: SEE SHEET 5

REVISED 1/00 BY FYC
CALCULATED J.L.G. CHECKED W.G.Y.

TYPICAL SECTIONS - PROPOSED U.S.R. 35 APPROACH SLABS

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\006.dgn 01-FEB-2000 2:44PM fcheek



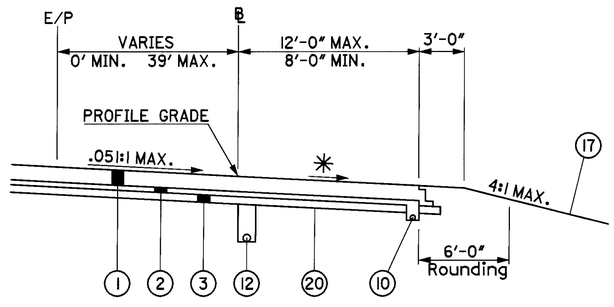
FOR TYPICAL DITCH
SEE SHEET 3

△ 0.083:1

(9) ITEM 304 DEPTH = 10"

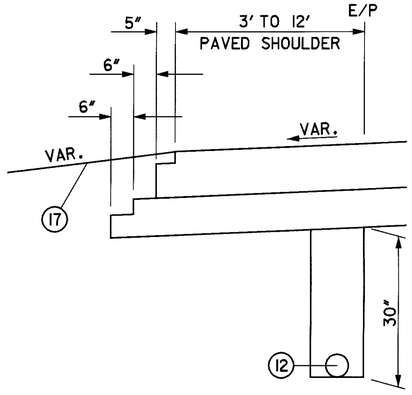
STA. 237+19.33 TO STA. 242+25, RAMP C = 505.67 L.F.

NORMAL RAMP SECTION

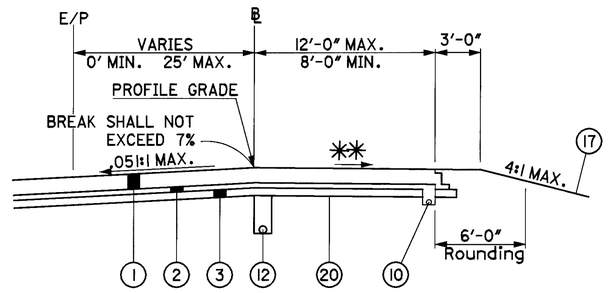


STA. 248+47.74 TO STA. 256+38.05, RAMP D = 790.31 L.F.

SPEED CHANGE LANE



RAMP
EDGE COURSE DETAIL
(TYP.)

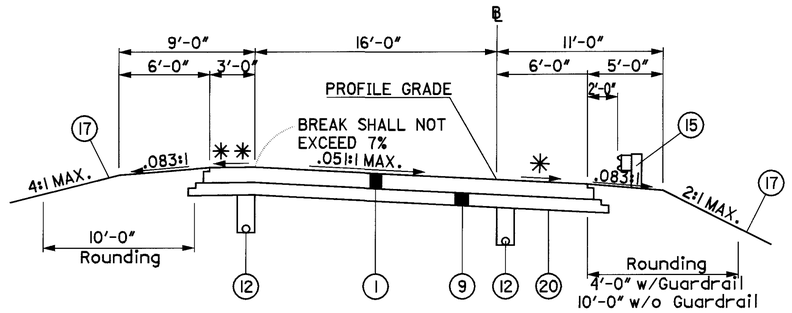


STA. 246+00 TO STA. 258+00, RAMP C = 1200 L.F.

SPEED CHANGE LANE

✳ SAME AS PAV'T.
.042:1 MIN.

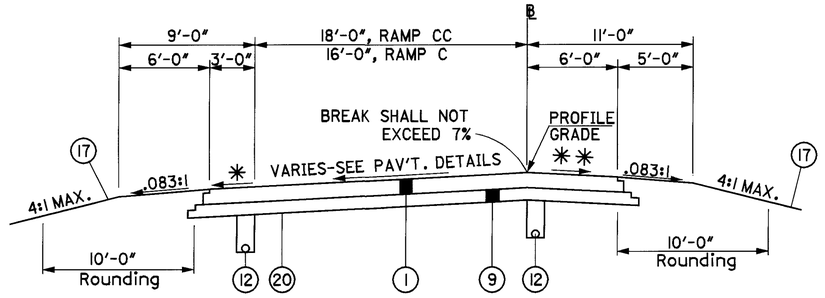
✳✳ VARIES 0.42:1 MAX.
TO .019:1 MIN.



(9) ITEM 304 DEPTH = 10"

STA. 227+48.46 TO STA. 230+59.33, RAMP C = 310.87 L.F. (SE-0.044)
STA. 237+22.03 TO STA. 248+47.74, RAMP D = 1125.71 L.F.

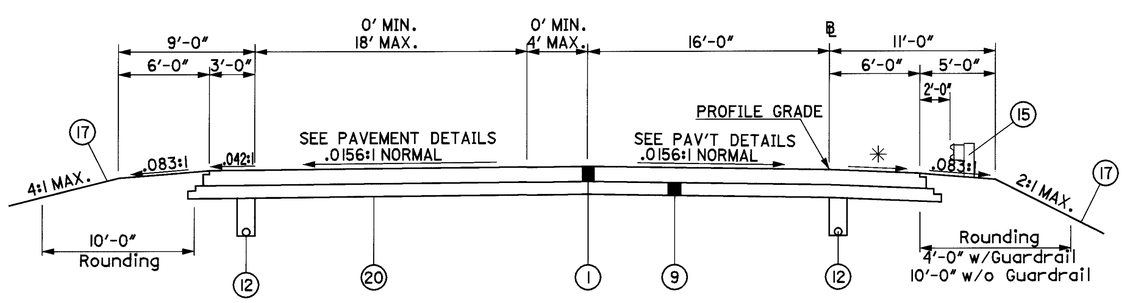
SUPERELEVATED RAMP SECTION



(9) ITEM 304 DEPTH = 10"

STA. 229+13.33 TO STA. 230+59.33, RAMP CC = 146.00 L.F.
STA. 242+25 TO STA. 245+00, RAMP C = 275.00 L.F.

SUPERELEVATED RAMP SECTION



(9) ITEM 304 DEPTH = 10"

STA. 230+59.33 TO STA. 237+19.33, RAMP C = 660.00 L. F.

NOTE: ALL TYPICAL SECTIONS ON THIS SHEET
ARE SHOWN IN THE DIRECTION OF TRAVEL.

FOR LEGEND: SEE SHEET 5

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 11/99 BY FYC

TYPICAL SECTIONS - RAMPS

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\fayramps.dgn 01-FEB-2000 2:57PM fcheek

I:\pr\project\fay35\fay-35-2.57\ fayramps.dgn

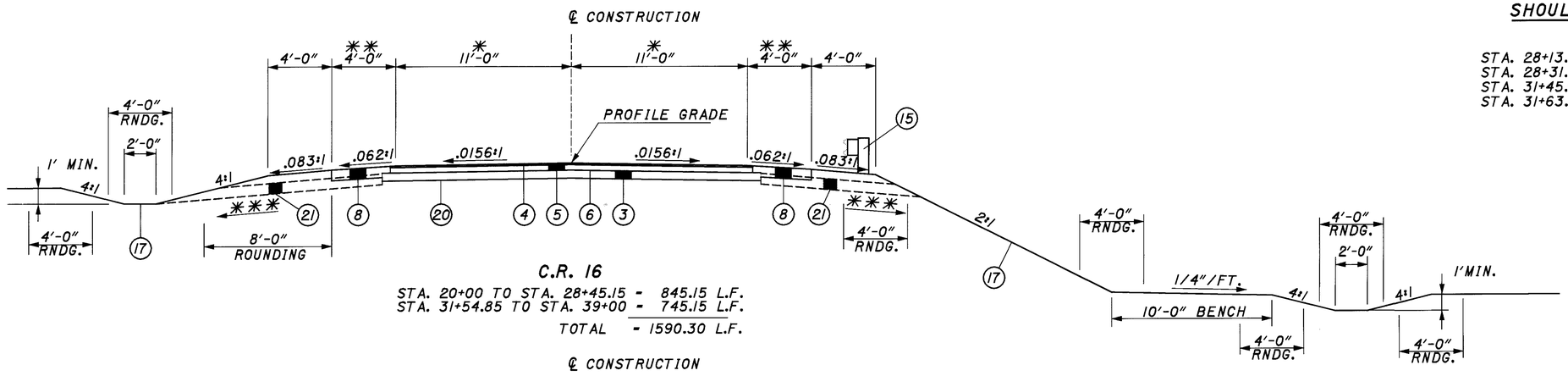
✳ **C.R. 16**
TAPER FROM 8.5′ AT STA. 20+00 TO 11′ AT STA. 21+37.5 LT.
TAPER FROM 11′ AT STA. 37+62.5 TO 8.5′ AT STA. 39+00 LT.
TAPER FROM 11′ AT STA. 38+50 TO 11.75′ AT STA. 39+00 RT.

✳ **T.R. 94**
TAPER FROM 10′ AT STA. 3+34 TO 8.8′ AT STA. 4+00 LT.& RT.

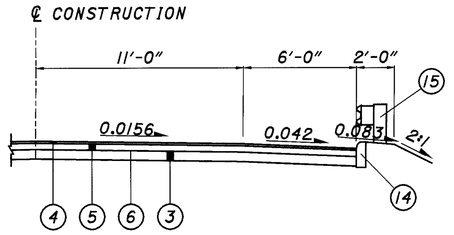
✳✳ **C.R. 16**
TAPER FROM 2′ AT STA. 20+00 TO 4′ AT STA. 20+50 LT.& RT.
TAPER FROM 4′ AT STA. 38+50 TO 2′ AT STA. 39+00 LT.& RT.

✳✳ **T.R. 94**
TAPER FROM 4′ AT STA. 3+34 TO 2′ AT STA. 4+00 LT.& RT.

✳✳✳ 1″/FT. DESIRABLE, ½″/FT. MIN.

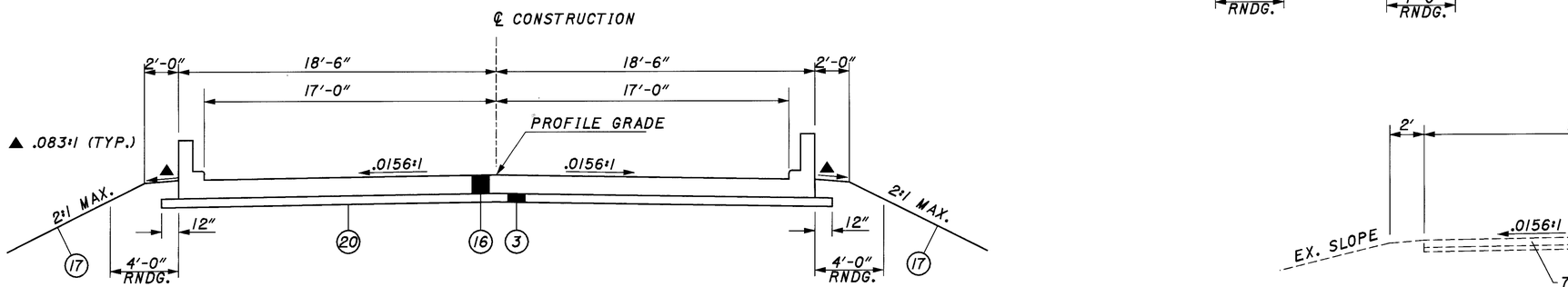


C.R. 16
STA. 20+00 TO STA. 28+45.15 - 845.15 L.F.
STA. 31+54.85 TO STA. 39+00 - 745.15 L.F.
TOTAL - 1590.30 L.F.



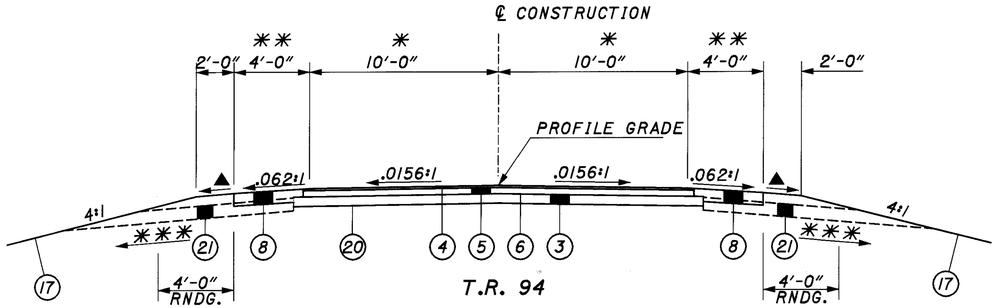
C.R. 16
SHOULDER DETAIL WITH CURB

STA. 28+13.25 TO STA. 28+36.25 RT. - 23 L.F.
STA. 28+31.32 TO STA. 28+54.32 LT. - 23 L.F.
STA. 31+45.68 TO STA. 31+68.68 RT. - 23 L.F.
STA. 31+63.75 TO STA. 31+86.75 LT. - 23 L.F.
TOTAL - 92 L.F.



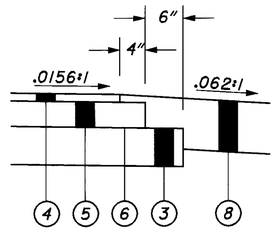
APPROACH SLAB C.R. 16

STA. 28+45.15 TO STA. 28+65.15 - 20.00 L.F. (T=13″)
STA. 31+34.85 TO STA. 31+54.85 - 20.00 L.F. (T=13″)
TOTAL - 40.00 L.F.

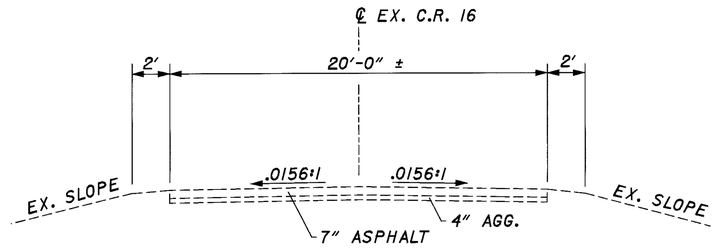


T.R. 94

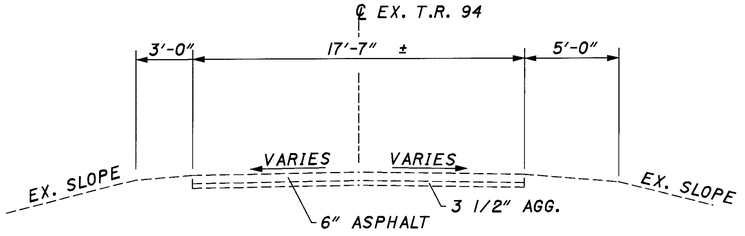
STA. 0+11 TO STA. 4+00 - 389.00 L.F.



C.R. 16 & T.R. 94
EDGE COURSE DETAIL



TYPICAL SECTION
EXISTING C.R. 16



TYPICAL SECTION
EXISTING T.R. 94

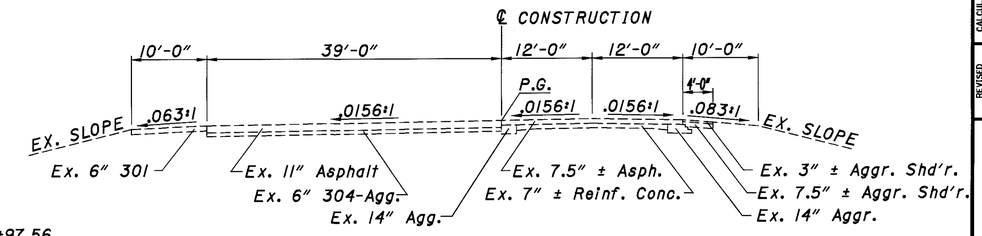
FOR LEGEND: SEE SHEET 5

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 1/00 BY FYC

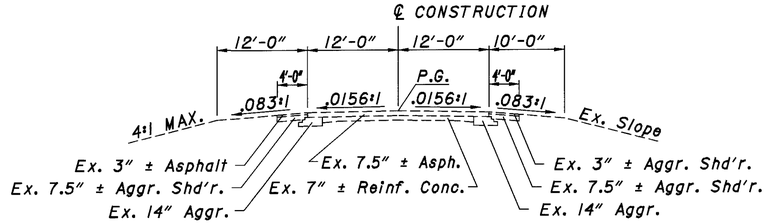
TYPICAL SECTIONS - C.R. 16 & T.R. 94

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\008.dgn 01-FEB-2000 2:50PM fcheek
I:\PR\PROJECT\FAY35\FAY-35-2.57\ 008.DGN



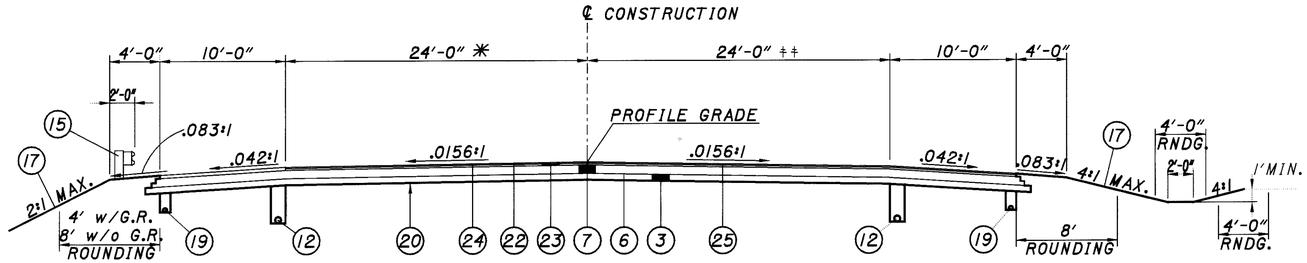
EX. PAV'T @ STA. 62+25



EX. PAV'T @ STA. 90+00

✳TAPER FROM 39' AT STA. 62+25 TO 24' AT STA. 68+85.

‡‡ TAPER FROM 24' AT STA. 69+87.48 TO 47' AT STA. 71+97.56

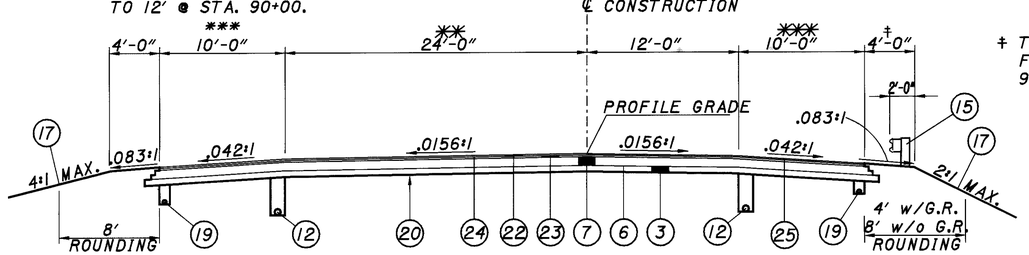


STA. 62+25 TO STA. 71+97.56 = 972.56 L.F.

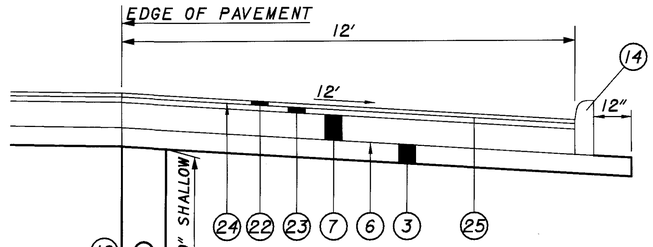
✳✳TAPER FROM 24' AT STA. 88+00 TO 12' @ STA. 90+00.

✳✳✳TAPER FROM 10' AT STA. 89+00 TO 4' @ STA. 90+00.

‡ TAPER IN A STRAIGHT LINE FROM 4' @ STA. 79+25 TO 9.07' @ STA. 84+07.77

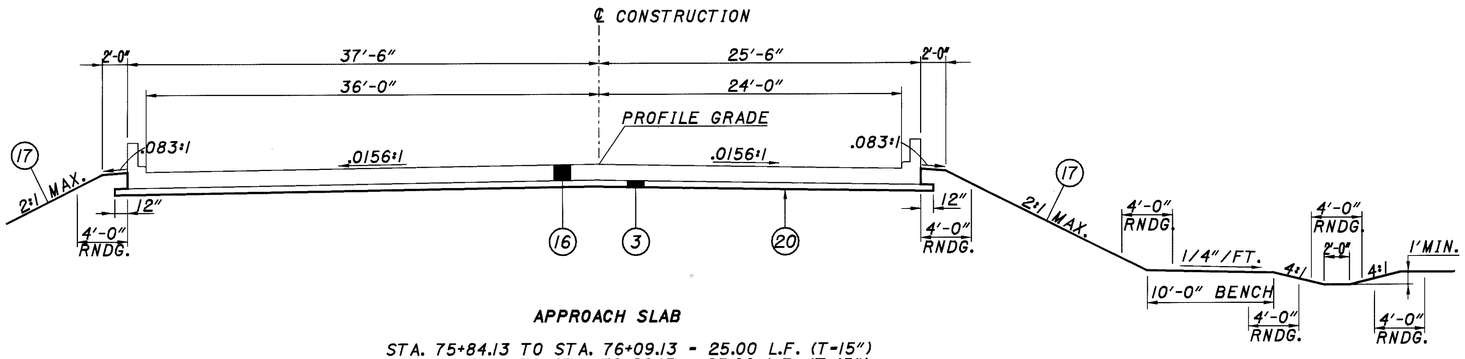


STA. 71+97.56 TO STA. 75+84.13 = 386.57 L.F.
STA. 78+86.13 TO STA. 90+00 = 1113.87 L.F.
TOTAL = 1500.44 L.F.



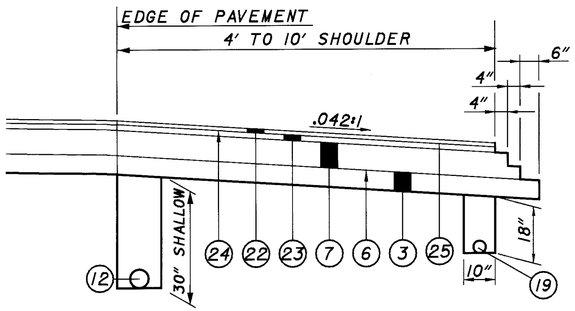
SHOULDER DETAIL WITH CURB

STA. 75+22.13 TO STA. 75+53.13 LT. = 31 L.F.
STA. 75+81.09 TO STA. 76+05.09 RT. = 24 L.F.
STA. 78+54.69 TO STA. 78+79.69 LT. = 25 L.F.
STA. 79+06.65 TO STA. 79+37.65 RT. = 31 L.F.
TOTAL = 111 L.F.



APPROACH SLAB

STA. 75+84.13 TO STA. 76+09.13 = 25.00 L.F. (T=15")
STA. 78+61.13 TO STA. 78+86.13 = 25.00 L.F. (T=15")
TOTAL = 50.00 L.F.



SHOULDER AND EDGE COURSE (TYP.)

NOTE: SHOULDER CROSS SLOPES SHALL BE TRANSITIONED TO THE PAVEMENT SLOPES WITHIN THE APPROACH SLAB LENGTH ON EACH SIDE OF THE STRUCTURE.

FOR LEGEND: SEE SHEET 5

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 1/00 BY FYC

TYPICAL SECTIONS - EXISTING U.S.R. 35

FAY-35-2.57

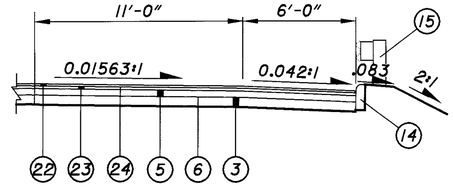
i:\pr\project\fay35\fay-35-2.57\009.dgn 01-FEB-2000 2:55PM fcheek

11\pr\project\fay35\fay-35-2.57\ 009.dgn

* TAPER FROM 9' AT STA. 152+75 TO 11' AT STA. 153+85 LT.
TAPER FROM 11' AT STA. 180+50 TO 13.6' AT STA. 181+44 LT.

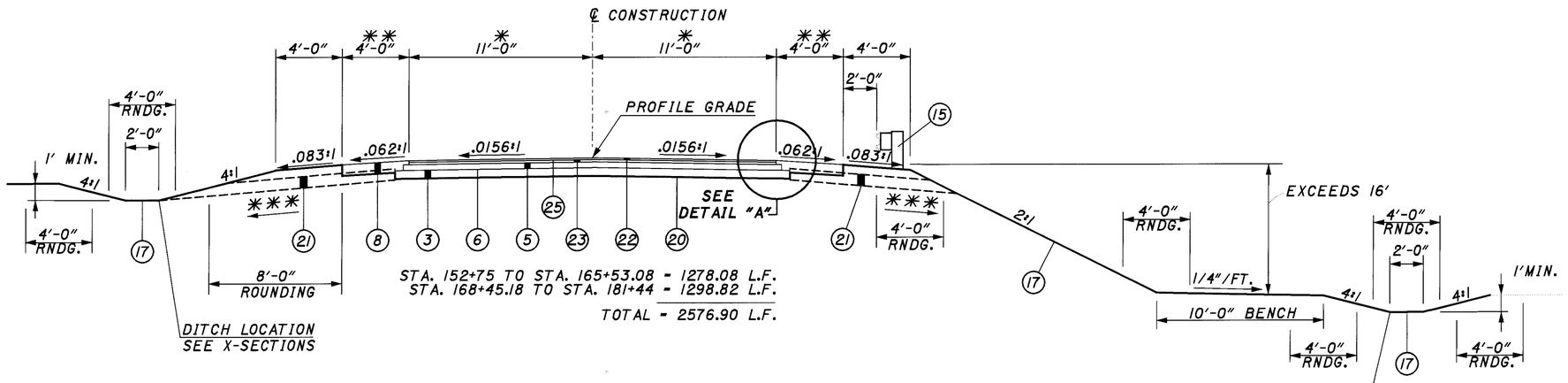
** TAPER FROM 3' AT STA. 152+75 TO 4' AT STA. 153+00 LT.&RT.

*** 1"/FT. DESIRABLE, ½"/FT. MIN.



SHOULDER DETAIL WITH CURB

STA. 165+23.91 TO STA. 165+47.91 RT. - 24 L.F.
STA. 165+34.27 TO STA. 165+58.27 LT. - 24 L.F.
STA. 168+40.00 TO STA. 168+64.00 RT. - 24 L.F.
STA. 168+50.37 TO STA. 168+74.37 LT. - 24 L.F.
TOTAL - 96 L.F.

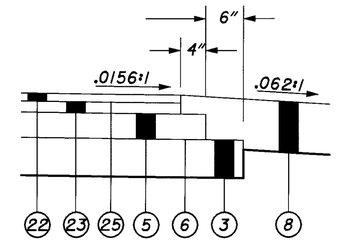


STA. 152+75 TO STA. 165+53.08 - 1278.08 L.F.
STA. 168+45.18 TO STA. 181+44 - 1298.82 L.F.
TOTAL - 2576.90 L.F.

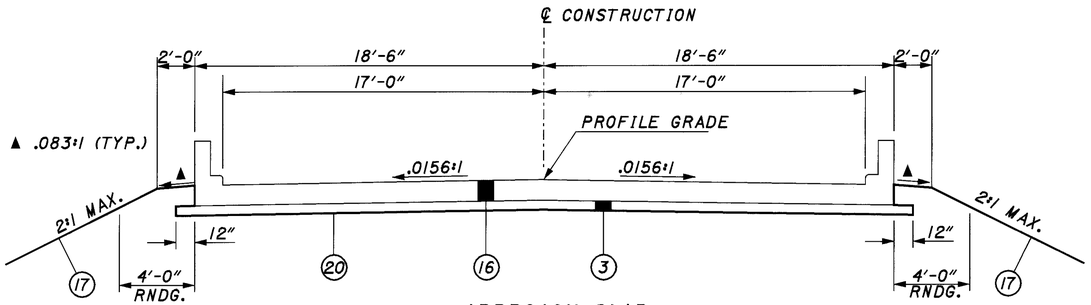
DEEP FILL SECTION

DEEP FILL SECTION

DITCH LOCATION
SEE X-SECTIONS



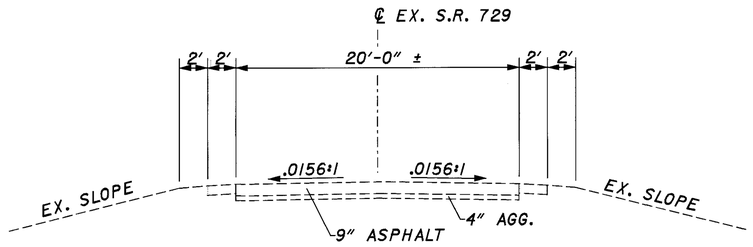
DETAIL "A"
S.R. 729
EDGE COURSE DETAIL



APPROACH SLAB

STA. 165+53.08 TO STA. 165+73.08 - 20.00 L.F. (T-13")
STA. 168+25.18 TO STA. 168+45.18 - 20.00 L.F. (T-13")
TOTAL - 40.00 L.F.

DEEP FILL SECTION



TYPICAL SECTION
EXISTING S.R. 729

FOR LEGEND: SEE SHEET 5

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 1/00 BY FYC

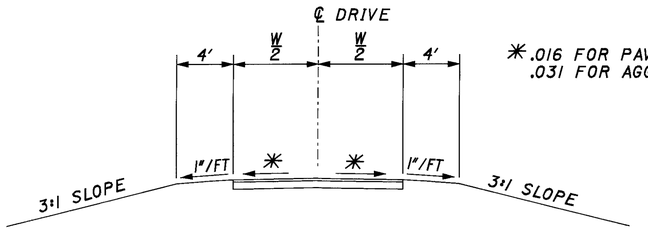
TYPICAL SECTIONS - S.R. 729

FAY-35-2.57

10
522

i:\pr\project\fay35\fay-35-2.57\010.dgn 01-FEB-2000 2:55PM fcheek

i:\pr\project\fay35\fay-35-2.57\ 010.dgn



✳ .016 FOR PAVED SURFACE.
.031 FOR AGGREGATE SURFACE.

TYPICAL SECTION
DRIVES

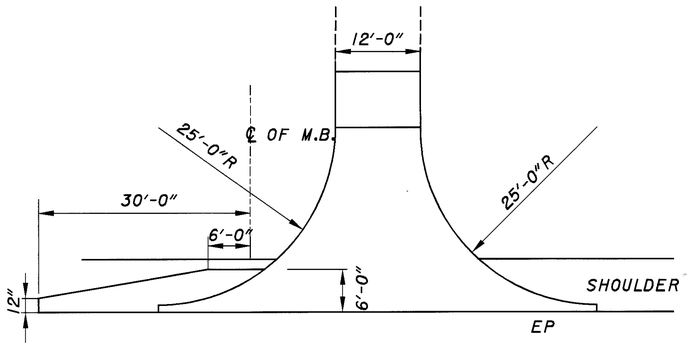
RES. DRIVES

FLARES:
ITEM 448 - 2" ASPHALT CONCRETE, SURFACE COURSE, TYPE I PG64-22, (DRIVEWAY)
ITEM 408 - BITUMINOUS PRIME COAT APPLIED AT THE RATE OF 0.40 GAL./S.Y.
ITEM 304 - 6" AGGREGATE BASE

BEYOND FLARES:
ITEM 304 - 8" AGGREGATE BASE

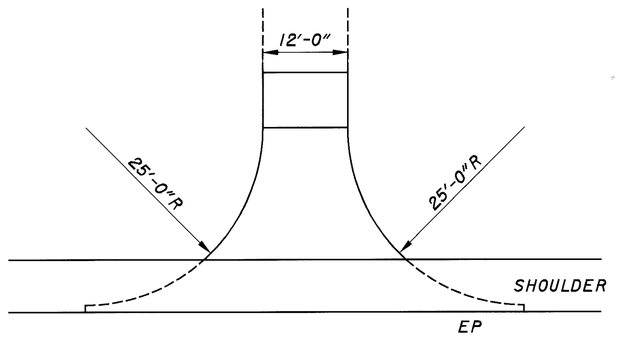
FIELD DRIVES

ITEM 304 - 6" AGGREGATE BASE



RES. DRIVES

NOTE: FOR MAIL BOX APPROACH
SEE STD. DWG. BP-4.1M

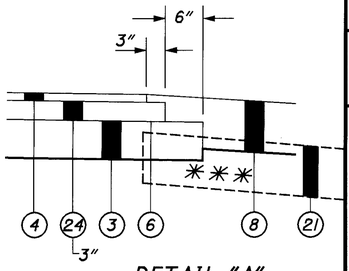


FIELD DRIVES

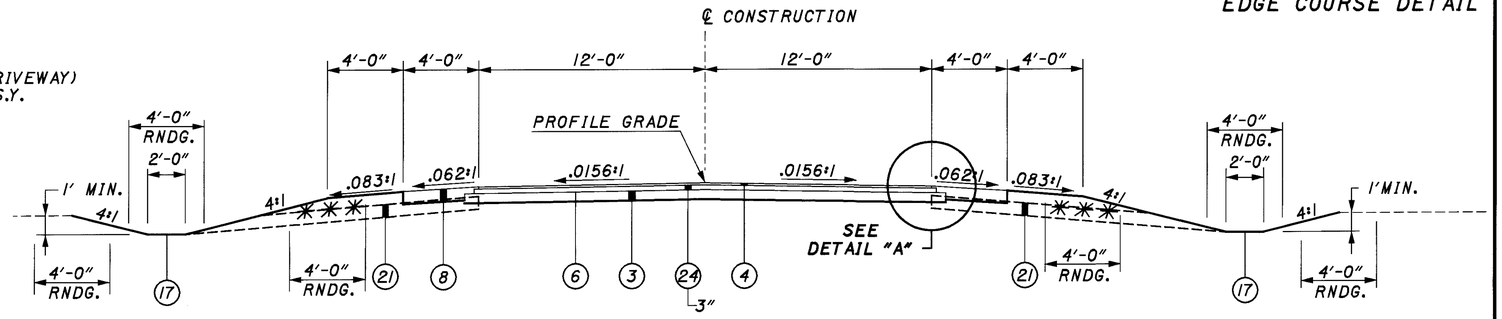
✳ SAME AS PAV'T.
.042:1 MIN.

✳✳ VARIES 0.42:1 MAX.
TO .019:1 MIN.

✳✳✳ 1"/FT. DESIRABLE, 1/2"/FT. MIN.

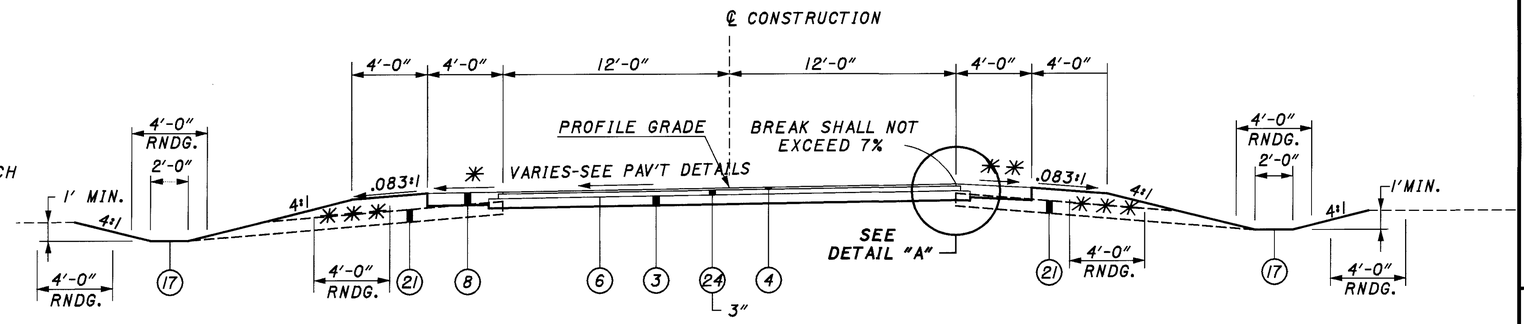


DETAIL "A"
SERVICE ROAD
EDGE COURSE DETAIL



NORMAL SERVICE ROAD SECTION

STA. 8+00.00 TO STA. 9+58.83 SERVICE RD. 2 - 158.83 L.F.
STA. 16+75.00 TO STA. 26+50.00 SERVICE RD. 3 - 975.00 L.F.



SUPERELEVATED SERVICE ROAD SECTION

STA. 10+34.52 TO STA. 13+00.00 SERVICE RD. 1 - 265.48 L.F.
STA. 26+50.00 TO STA. 29+88.00 SERVICE RD. 3 - 338.00 L.F.

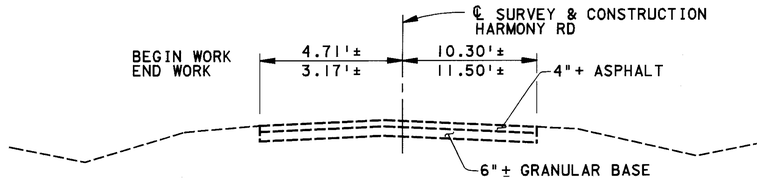
FOR LEGEND: SEE SHEET 5

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 1/00 BY FYC

TYPICAL SECTIONS - SERVICE ROADS AND DRIVEWAYS

FAY-35-2.57

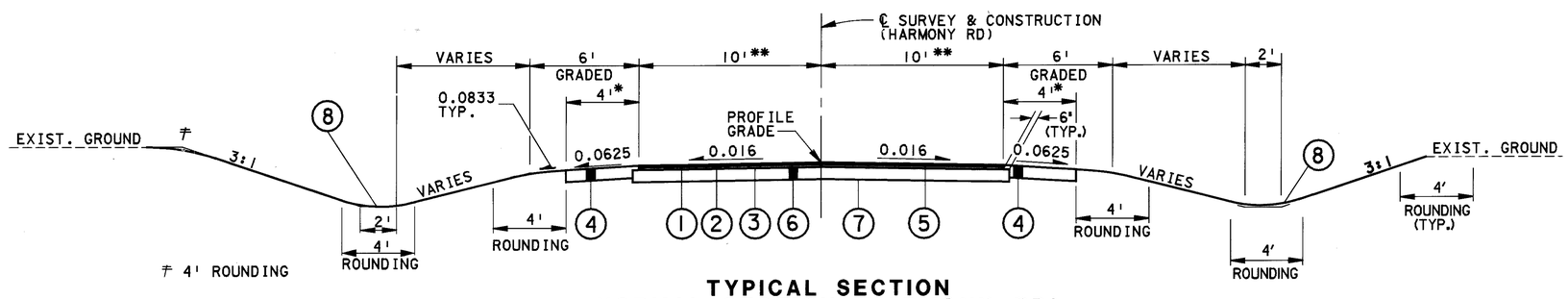
I:\pr\project\fay35\fay-35-2.57\011.dgn 01-FEB-2000 2:51PM fcheek
I:\pr\project\fay35\fay-35-2.57\ 011.dgn



TYPICAL SECTION
EXISTING SECTION HARMONY RD

LEGEND

NO.	ITEM	DESCRIPTION
(1)	448	1¼" ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
(2)	448	1¾" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
(3)	408	BITUMINOUS PRIME COAT (0.4 GAL./S.Y.)
(4)	411	8" STABILIZED CRUSHED AGGREGATE
(5)	407	TACK COAT FOR INTERMEDIATE COURSE
(6)	304	8" AGGREGATE BASE
(7)	203	SUBGRADE COMPACTION
(8)	659	SEEDING AND MULCHING



TYPICAL SECTION
NORMAL SECTION (HARMONY RD)

** TAPER FROM 4.7' AT STA. 24+50, TO 10' AT STA. 25+63 (LT.)
TAPER FROM 10.0' AT STA. 29+24, TO 3.1' AT STA. 30+44 (LT.)

TAPER FROM 10.3' AT STA. 24+50, TO 10.0' AT STA. 25+63 (RT.)
TAPER FROM 10.0' AT STA. 29+24, TO 11.5 AT STA. 30+44 (RT.)

* TAPER FROM 2.0' AT STA. 25+06.56 (L/R), TO 4.0' AT STA. 25+63 (L/R)
TAPER FROM 4.0' AT STA. 29+24 (L/R). TO 2.0' AT STA. 29+84.71 (L/R)

CALCULATED JPC | CHECKED KDO
REVISED 11/99 | BY FYC

TYPICAL SECTIONS - HARMONY ROAD

FAY-35-2.57

i:\pr\project\fay35\fay-35-2.57\ gyharmony.dgn

ROUNDING

THE ROUNDING AT SLOPE BREAKPOINTS SHOWN ON THE TYPICAL SECTIONS APPLY TO ALL CROSS-SECTIONS EVEN THOUGH OTHERWISE SHOWN.

UTILITIES

LISTED BELOW ARE ALL UTILITIES LOCATED WITHIN THE PROJECT CONSTRUCTION LIMITS TOGETHER WITH THEIR RESPECTIVE OWNERS:

DAYTON POWER AND LIGHT CO.
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937) 331-4498

FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(740) 335-1541

DUKE ENERGY
5444 WESTHEIMER
WT 610
P.O. BOX 1642
HOUSTON, TEXAS 77251-1642
(713) 989-2232

CNG TRANSMISSION CORP.
P.O. BOX 2450
CLARKSBURG, WV 26302-2450
(304) 623-8454

AT&T COMMUNICATIONS
3972 BROWN PARK DRIVE
SUITE D
HILLIARD, OHIO 43026
(614) 876-9366

AMERITECH
150 E. GAY STREET, ROOM 6C
COLUMBUS, OHIO 43215
(614) 223-8535

FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740) 775-4288

TEXAS EASTERN PRODUCTS PIPELINE CO.
P.O. BOX 426
SEYMOUR, INDIANA 47274-0426
(812) 522-2569

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS AS REQUIRED BY SECTION 153.64 O.R.C.

CONTINGENCY QUANTITIES

THE CONTRACTOR SHALL NOT ORDER MATERIALS OR PERFORM WORK FOR ITEMS DESIGNATED BY PLAN NOTE TO BE USED "AS DIRECTED BY THE ENGINEER" UNLESS AUTHORIZED BY THE ENGINEER. THE ACTUAL WORK LOCATIONS AND QUANTITIES USED FOR SUCH ITEMS SHALL BE INCORPORATED INTO THE FINAL ORDER GOVERNING COMPLETION OF THIS PROJECT.

TEMPORARY STREAM CROSSING FORDS

WHERE STREAM CROSSING FORDS ARE REQUIRED FOR EQUIPMENT CROSSING, THE CROSSING SHALL CONSIST OF CLEAN NON-TOXIC GRANULAR OR ROCK MATERIAL, PROPERLY MAINTAINED TO PREVENT EROSION, WITH PROVISIONS FOR CONVEYANCE OF ANTICIPATED HIGH FLOWS AND SHALL NOT IMPEDE THE MOVEMENT OF AQUATIC LIFE. CONSTRUCTION SHALL BE IN ACCORDANCE WITH PART 330, APPENDIX A, SPECIFIC CATEGORIES OF DISCHARGES-NATIONALLY PERMITTED, PARAGRAPH (A14), MINOR ROAD CROSSING FILLS - THE FEDERAL REGISTER - CORPS OF ENGINEERS FINAL REGULATIONS, CURRENT EDITION.

ELEVATION DATUM

ALL ELEVATIONS ARE BASED ON U.S.G.S. BENCHMARKS V-36 AND X-36.

REMOVAL OF TREES OR STUMPS

ALL TREES AND STUMPS SPECIFICALLY MARKED FOR REMOVAL WITHIN THE CONSTRUCTION LIMITS SHALL BE REMOVED UNDER THE LUMP SUM BID FOR ITEM 201, CLEARING AND GRUBBING. THE FOLLOWING IS AN APPROXIMATE ESTIMATE OF THE NUMBER OF TREES AND STUMPS TO BE REMOVED:

SIZES	NO. TREES	NO. STUMPS	TOTAL
18"	67	6	73
30"	8	3	11
48"	0	3	3
60"	0	0	0

MONUMENTS

MONUMENTS SHALL BE CONTRUCTED IN ACCORDANCE WITH DETAILS AS SHOWN ON THE STANDARD DRAWINGS AND AT THE LOCATIONS SHOWN ON SHEET NO. 483 & NO. 484.

ITEM 203, PROOF ROLLING

AN ESTIMATED QUANTITY FOR THIS ITEM HAS BEEN PROVIDED IN THE GENERAL SUMMARY FOR USE AS DIRECTED BY THE ENGINEER.

ITEM 606 - ANCHOR ASSEMBLY, TYPE E-98

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING EITHER OF THE FOLLOWING GUARDRAIL END TERMINALS.

1) THE ET-2000 (1997), MANUFACTURED BY SYRO INC., 1170 N. STATE STREET, GIRARD, OHIO 44420 (TELEPHONE: 330-545-4373).

THE LENGTH OF THE ET-2000 (1997) SYSTEM IS CONSIDERED TO BE 50'-0", INCLUSIVE OF TWO 25'-0" LONG RAIL ELEMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
SS265M	ET-2000 (1997) PLAN, ELEVATION & SECTIONS	6/20/97	3/6/98

2) THE SKT-350 MANUFACTURED BY ROAD SYSTEMS, INC., 7631 NEW CASTLE DRIVE, FRANKFORT, IL 60423 (TELEPHONE: 815-464-5917).

THE LENGTH OF THE SKT-350 SYSTEM IS CONSIDERED TO BE 50'-0", INCLUSIVE OF FOUR 12'-6" LONG RAIL ELEMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
SKT-4M	SEQUENTIAL KINKING TERMINAL (SKT-350) ASSEMBLY WITH 4 FOUNDATION TUBES	12/11/97	3/6/98

THE FACE OF THE TYPE E-98 IMPACT HEAD SHALL BE COVERED WITH A SHEET OF TYPE G REFLECTIVE SHEETING, PER CMS 730.19, APPROXIMATELY 18"x18".

PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 606, ANCHOR ASSEMBLY, TYPE E-98, EACH, AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT A COMPLETE AND FUNCTIONAL ANCHOR ASSEMBLY SYSTEM, INCLUDING ALL RELATED TRANSITIONS, REFLECTIVE SHEETING, HARDWARE, GRADING, EMBANKMENT AND EXCAVATION NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

ITEM 606 - ANCHOR ASSEMBLY, TYPE B-98

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING EITHER OF THE FOLLOWING GUARDRAIL END TERMINALS.

1) THE SRT-350, GUARDRAIL END TERMINAL AS MANUFACTURED BY SYRO INC., 1170 N. STATE STREET, GIRARD, OHIO 4420 (TELEPHONE: 330-545-4373)

THE LENGTH OF THE SRT-350 SYSTEM IS CONSIDERED TO BE 37'-6", INCLUSIVE OF THREE 12'-6" LONG RAIL ELEMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
SS444 SS444M	SLOTTED RAIL TERMINAL POST LAYOUT AND ERECTION DETAILS SRT-350 (12.5, 8 POST)	7/12/99 REV. 7/12/99	8/27/99
SS425M	SLOTTED RAIL TERIMNAL SRT-350 POST LAYOUT AND ERECTION DETAILS (12.5, 9 POST)	6/21/97	3/6/98

2) THE FLEAT-350 MANUFACTURED BY ROAD SYSTEMS, INC., 7631 NEW CASTLE DRIVE, FRANKFORT, IL 60423 (TELEPHONE: 815-464-5917).

THE LENGTH OF THE FLEAT-350 IS CONSIDERED TO BE 37'-6", INCLUSIVE OF THREE 12'-6" LONG RAIL ELEMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
FLT-M	FLARED ENERGY ABSORBING TERMINAL (FLEAT-350) ASSEMBLY	4/16/98	7/13/98

GRADING SHALL BE IN ACCORDANCE WITH STANDARD CONSTRUCTION DRAWING GR-4.3.

THE FACE OF THE TYPE B-98 IMPACT HEAD SHALL BE COVERED WITH TYPE G REFLECTIVE SHEET, PER CMS 730.19: APPROXIMATELY 36"Wx12"H FOR THE SRT-350 AND 14"Wx20"H FOR THE FLEAT.

PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 606 ANCHOR ASSEMBLY, TYPE B-98, EACH, AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT A COMPLETE AND FUNCTIONAL ANCHOR ASSEMBLY SYSTEM, INCLUDING REFLECTIVE SHEETING AND ALL RELATED HARDWARE, GRADING, EMBANKMENT AND EXCAVATION NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

ITEM 606, IMPACT ATTENUATOR, TYPE I-98 (BIDIRECTIONAL)

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING EITHER OF THE FOLLOWING IMPACT ATTENUATORS:

1) THE C-A-T MANUFACTURED BY SYRO, INC., 1170 N. STATE STREET GIRARD, OHIO 44420 (TELEPHONE: 330-545-4373). THE LENGTH OF THE C-A-T SYSTEM IS CONSIDERED TO BE 31.25 FEET LONG. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
SS245M	CRASH-CUSHION ATTENUATING TERMINAL PLAN, ELEVATION & SECTIONS FOR USE AS A LONGITUDINAL MEDIAN BARRIER TERMINAL OR CRASH CUSHION ATTENUATOR	4/10/97 Rev. 4	3/6/98
SS224M	C-A-T TRANSITION TO MEDIAN BARRIER GUARDRAIL PLAN, ELEVATION & SECTIONS	4/26/96	3/6/98
SS226M	C-A-T TRANSITION TO VERTICAL WALL OR PIER PLAN, ELEVATION & SECTIONS	4/26/96	3/6/98

2) THE BRAKEMASTER MANUFACTURED BY ENERGY ABSORPTION SYSTEMS, INC., ONE EAST WACKER DRIVE, CHICAGO, IL 60601 (TELEPHONE: 312-467-6750). THE LENGTH OF THE BRAKEMASTER SYSTEM IS CONSIDERED TO BE 31.25 FEET LONG. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AS DETAILED ON THE FOLLOWING PRE-APPROVED SHOP DRAWINGS:

DWG.#	DRAWING NAME	DWG./REV. DATE	ODOT APPROVAL DATE
92-00-01	BRAKEMASTER GENERAL ASSEMBLY (UNIDIRECTIONAL SYSTEM)	3/6/97 Rev. K	3/6/98
92-00-81	BRAKEMASTER (UNIDIRECTIONAL) WITH FOUNDATION TUBES	2/9/98	3/6/98
92-00-02	BRAKEMASTER GENERAL ASSEMBLY (BIDIRECTIONAL SYSTEM)	3/10/97 Rev. K	3/6/98
92-00-82	BRAKEMASTER (BIDIRECTIONAL) WITH FOUNDATION TUBES	2/9/98	3/6/98
9202024-0000	ANCHOR ASSEMBLY, FOUNDATION TUBE, 6-1/2 FT., BRS	6/12/97 Rev. D	3/6/98

THE FACE OF THE TYPE I-98 IMPACT HEAD SHALL BE COVERED WITH A SHEET OF TYPE G REFLECTIVE SHEETING, PER CMS 730.19, APPROXIMATELY 36"x12". PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 606, IMPACT ATTENUATOR, TYPE I-98 (BIDIRECTIONAL), EACH, AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT A COMPETED AND FUNCTIONAL ANCHOR ASSEMBLY SYSTEM, INCLUDING ALL RELATED TRANSITIONS, HARDWARE, REFLECTIVE SHEETING AND GRADING, NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

ITEM 870, SEEDING AND MULCHING

SEEDING AND MULCHING SHALL BE APPLIED TO ALL AREAS OF EXPOSED SOIL BETWEEN THE RIGHT-OF-WAY LINES, AND WITHIN THE CONSTRUCTION LIMITS FOR AREAS OUTSIDE THE RIGHT-OF-WAY LINES COVERED BY WORK AGREEMENT OR SLOPE EASEMENT. QUANTITY CALCULATIONS FOR ITEM 870, SEEDING AND MULCHING ARE BASED ON THESE LIMITS.

THE FOLLOWING ESTIMATED QUANTITIES ARE TO BE USED AS DIRECTED BY THE ENGINEER TO PROMOTE GROWTH AND TO CARE FOR PERMANENT SEEDED AREAS AS PER 870.14 :

870, WATER	4421 M. GAL
870, MOWING	1842 M. SQ. FT.
870, SOIL ANALYSIS TEST	17 EACH
870, PLACING TOPSOIL	45516 CU.YD.

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 10/99 BY FYC

GENERAL NOTES

FAY-35-2.57

Q:\3590\3590GN01

ITEM SPECIAL - MAILBOX SUPPORT

THIS WORK SHALL CONSIST OF FURNISHING AND ERECTING MAILBOX SUPPORTS AND ANY ASSOCIATED MOUNTING HARDWARE IN ACCORDANCE WITH PLAN DETAILS, AND ATTACHING AN OWNER-SUPPLIED MAILBOX AT LOCATIONS SPECIFIED IN THE PLAN, OR OTHERWISE ESTABLISHED BY THE ENGINEER.

WOOD POSTS SHALL BE NOMINAL 4" X 4" SQUARE OR 4-1/2" DIAMETER ROUND, AND CONFORM TO 710.14.

STEEL POSTS SHALL BE NOMINAL PIPE SIZE 2" I.D., AND CONFORM TO AASHTO M 181.

HARDWARE (PLATES, SCREWS, BOLTS, ETC.) SHALL BE COMMERCIAL-GRADE GALVANIZED STEEL.

POSTS SHALL BE SET PER THE FIRST PARAGRAPH OF 606.03, AND SHALL IN NO INSTANCE BE ENCASED IN CONCRETE.

SUPPORT HANDWARE SHALL ACCOMMODATE EITHER A SINGLE OR A DOUBLE MAILBOX INSTALLATION, AND NO MORE THAN TWO BOXES MAY BE MOUNTED ON A SINGLE POST.

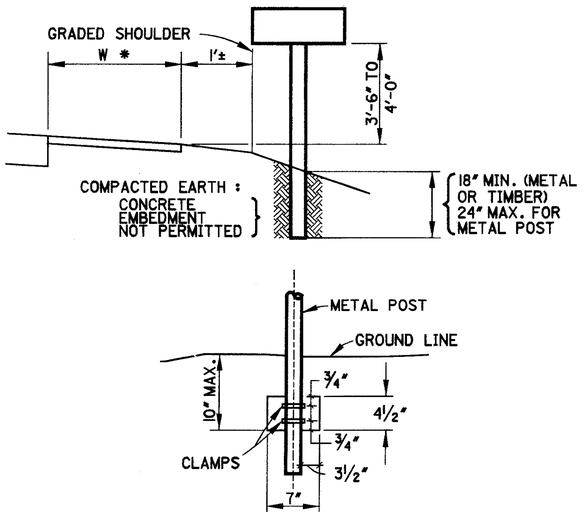
THE MAILBOX SHALL BE SECURELY AND NEATLY ATTACHED BY THE CONTRACTOR TO THE NEW SUPPORT. THE CONTRACTOR SHALL FURNISH ALL NECESSARY ATTACHMENT HARDWARE (NUTS, BOLTS, PLATES, SPACERS AND WASHERS) AS NECESSARY TO ACCOMMODATE THE COMPLETE INSTALLATION.

IN THE ABSENCE OF A NEW BOX SUPPLIED BY THE OWNER, THE CONTRACTOR SHALL SALVAGE THE EXISTING BOX AND PLACE IT ON THE NEW SUPPORT. DUE CARE SHALL BE EXERCISED IN SUCH AN OPERATION, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING OR REPLACING ANY BOX DAMAGED BY IMPROPER HANDLING ON HIS PART, AS JUDGED AND DIRECTED BY THE ENGINEER.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH THE LOCAL POST MASTER REGARDING THE TIMING OF THE MOVEMENT OF ANY MAILBOX TO A NEW LOCATION.

PAYMENT UNDER THIS ITEM SHALL BE LIMITED TO FINAL PERMANENT INSTALLATIONS. TEMPORARY INSTALLATIONS SHALL BE IN ACCORDANCE WITH 107.12. HOWEVER, THE SAME MATERIAL AND SIZE LIMITATIONS AS FOR PERMANENT INSTALLATIONS SHALL APPLY.

MAILBOX SUPPORTS, COMPLETE IN PLACE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER EACH, FOR ITEM SPECIAL MAILBOX SUPPORT (SINGLE) (DOUBLE).



ANTI-TWIST PLATE

REVISED PAVED SHOULDER DIMENSIONS

DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12FT (BOTH SIDES) TO 10FT (OUTSIDE) AND 6FT (INSIDE).

PAVED SHOULDER DIMENSIONS SHOWN ON THE TYPICAL SECTIONS, PAVEMENT CALCULATIONS, PAVEMENT DETAILS, SIGNING DETAILS, AND CULVERT DETAILS HAVE BEEN REVISED. PAVED SHOULDER DIMENSIONS SHOWN ON THE PLAN/PROFILE SHEETS, CROSS SECTIONS, MAINTENANCE OF TRAFFIC SHEETS, TRAFFIC CONTROL SHEETS, STORM WATER POLLUTION PREVENTION PLAN SHEETS, AND ALL OTHER RELATED MATERIALS HAVE NOT BEEN REVISED.

CONVERSION OF STANDARD CONSTRUCTION DRAWINGS

THE METRIC STANDARD DRAWINGS REFERENCED IN THIS PLAN SHALL BE CONVERTED TO ENGLISH UNITS USING THE SI (METRIC) TO ENGLISH CONVERSION FACTORS PROVIDED IN SECTION 109.11 OF THE 1997 CONSTRUCTION AND MATERIALS SPECIFICATIONS. THE APPENDIX OF ASTM E 380 SHALL BE UTILIZED FOR ANY ADDITIONAL CONVERSION FACTORS REQUIRED. CONVERSIONS SHALL BE APPROPRIATELY PRECISE AND SHALL REFLECT STANDARD INDUSTRY ENGLISH VALUES WHERE SUITABLE.

ITEM 607 - FENCE MISC.: CONSTRUCTION FENCE

THE CONSTRUCTION FENCE SHALL CONSIST OF ORANGE ROLL FENCING OR APPROVED EQUAL, AND SHALL BE USED AS PROTECTION TO KEEP INDIVIDUALS FROM FALLING INTO SEDIMENT BASINS. THE FENCE SHALL BE SUPPORTED BY STEEL OR WOOD POSTS OF ADEQUATE SIZE AND SPACING. USED FENCING AND POSTS IN GOOD CONDITION MAY BE USED AS APPROVED BY THE ENGINEER. ALL PLACEMENT LOCATIONS OF THE CONSTRUCTION FENCE SHALL BE APPROVED BY THE ENGINEER. PAYMENT WILL BE AT THE LUMP SUM PRICE BID FOR, ITEM 607 - FENCE MISC.: CONSTRUCTION FENCE, FURNISHED, ERECTED, AND SUBSEQUENTLY REMOVED.

REVISED STRUCTURE SHEETS

THE OFFICE OF STRUCTURAL ENGINEERING HAS MADE REVISIONS TO THE FOLLOWING SHEETS: 395, 396, 403, 404, 406, 407, 411, 412, 416, 419, 420, 428, 429, 436 THRU 440, 448, 454, 455, 457, 458, 462, 463, 466, 469, 470, 476, 478, AND 482. SEE TITLE SHEET FOR THE ENGINEER'S REGISTRATION NUMBER AND SIGNATURE.

JURISDICTIONAL WATERS OF THE US

THE CONTRACTOR IS PROHIBITED TO WORK WITHIN RATTLESNAKE CREEK (STA. 138+00) AND THE WETLAND SITE C (STA. 258+00) UNTIL SUCH TIME AS THE APPROVAL OF THE DEPARTMENT OF THE ARMY PERMIT UNDER SECTION 404 OF THE CLEAN WATER ACT HAS BEEN GRANTED. IT IS ANTICIPATED THAT A PERMIT WILL BE ISSUED BY JANUARY 1, 2001.

THE CONTRACTOR WILL BE NOTIFIED AND MADE AWARE OF ANY SPECIAL CONDITIONS ONCE A PERMIT HAS BEEN GRANTED.

ITEM SPECIAL, ENVIRONMENTALLY-SENSITIVE SITES

STUDIES DURING EARLIER STAGES OF PROJECT DEVELOPMENT IDENTIFIED ENVIRONMENTALLY-SENSITIVE SITES IN THE AREA. IN ADDITION, OTHER SITES MAY EXIST THAT WERE NOT INVENTORIED BY THOSE STUDIES. ENVIRONMENTALLY-SENSITIVE SITES ARE THOSE THAT POSSESS ATTRIBUTES OR RESOURCES AFFORDED CERTAIN PROTECTIONS UNDER VARIOUS FEDERAL, STATE, AND LOCAL REGULATIONS (SUCH AS WETLANDS, HISTORIC PROPERTIES, ARCHAEOLOGICAL SITES, AND SPECIAL HABITATS). MAPPING AND/OR REPORTS THAT DOCUMENT THE LOCATIONS OF THE SITES THAT HAVE ALREADY BEEN IDENTIFIED, ARE AVAILABLE FOR REVIEW AT THE ODOT DISTRICT 6 HEADQUARTERS, ENVIRONMENTAL STUDIES SECTION. CONTACT JEFFREY WHITE AT (800) 372-7714, EXTENSION 356.

ANY LOCATION PROPOSED BY THE CONTRACTOR FOR ANY ANCILLARY CONSTRUCTION PURPOSE (SUCH AS: FIELD OFFICE LOCATION, PARKING, STORAGE, BORROW OR WASTE AREAS) THAT IS NOT WITHIN THE WORK LIMITS, SHALL FIRST BE INVESTIGATED BY THE CONTRACTOR FOR ENVIRONMENTAL SENSITIVITY. ANY LOCATIONS THAT ARE DETERMINED TO BE SENSITIVE SHALL NOT BE DISTURBED, UNLESS PRIOR APPROVAL IS OBTAINED FROM THE APPROPRIATE CONTROLLING AGENCY/OFFICE IN COMPLIANCE WITH ALL RELEVANT FEDERAL, STATE, AND LOCAL LAWS.

THERE COULD BE CULTURAL SITES THAT ARE ELIGIBLE FOR THE NATIONAL REGISTER OF HISTORIC PLACES. THE CONTRACTOR SHALL NOT DISTURB ANY SUCH SITES UNLESS THOSE SITES ARE PROFESSIONALLY SURVEYED, EVALUATED, AND DOCUMENTED, AND CLEARANCE IS MADE THROUGH THE OHIO HISTORIC PRESERVATION OFFICE (OHPO). IF SUCH TASKS BECOME NECESSARY, THE CONTRACTOR IS RESPONSIBLE FOR ENGAGING AN CULTURAL RESOURCES CONSULTING FIRM ACCEPTABLE TO ODOT AND OHPO.

ITEM SPECIAL - INDIANA BAT HABITAT

CLEARING OF ANY TREES THAT SUITABLE SUMMER BROODREARING OR ROOSTING HABITAT FOR THE FEDERALLY ENDANGERED INDIANA BAT (e.g. TREES WITH A DBH OF 9 INCHES OR GREATER, EXFOLIATING BARKAND/OR CAVITIES), SHALL OCCUR DURING THE PERIOD BETWEEN SEPTEMBER 16 AND APRIL 14, WHEN TH EBATS WOULD NOT BE USING SUCH HABITAT.

SPECIAL CONDITIONS

THIS PROJECT AS DESIGNED COMPLIES WITH THE REQUIREMENTS AND CONDITIONS SET BY THE FAYETTE COUNTY FLOOD PLAIN MANAGER AND THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA). THE FAYETTE COUNTY FLOOD PLAIN MANAGER ISSUED APPROVAL ON NOVEMBER 3, 1999.

REVISED	CALCULATED
1/00	J.L.G.
BY	CHECKED
FYC	W.G.Y.

GENERAL NOTES

FAY-35-2.57

12A
522

Revised 7/26/00 By: E.R.P.

i:\pr\project\fay35\fay-35-2.57\012A.dgn 01-FEB-2000 2:38PM fcheek

i:\pr\project\fay35\fay-35-2.57\ 012a.dgn

NATIVE TREE PLANTING AREAS

DECIDUOUS TREES NATIVE TO THE RATTLESNAKE CREEK WATERSHED SHALL BE PLANTED IN THE AREAS AS SHOWN ON SHEET 54 AND IN THOSE AREAS WHERE TREES HAVE BEEN REMOVED TO FACILITATE CONSTRUCTION. THE TREES SHALL BE PLANTED IN LOCATIONS "AS DIRECTED BY THE ENGINEER" AND HAVE A DENSITY OF APPROXIMATELY ONE (1) TREE PER 100 SQ.FT. ALL REQUIREMENTS OF ITEM 661 'PLANTING TREES, SHRUBS AND VINES' AND ITEM 662 'LANDSCAPE WATERING' SHALL APPLY TO THIS WORK.

A MINIMUM OF THREE DIFFERENT VARIETIES SHALL BE PLANTED IN EACH AREA FROM THE LIST BELOW.

ITEM 661 PLANTING, MISC.: DECIDUOUS TREE, 2'-3' HEIGHT 225 EACH

BOX ELDER	(ACER NEGUNDO)
SILVER MAPLE	(ACER SACCHARINUM)
GREEN ASH	(FRAXINUS PENNSYLVANICA LANCEOLATA)
SYCAMORE	(ACER PSEUDOPLATANUS)
COTTONWOOD	(POPULUS)
WHITE WILLOW	(SALIX ALBA)

CONDITIONS OF THE OEPA 401 CERTIFICATION

1. STEPS SHALL BE TAKEN DURING CONSTRUCTION TO MINIMIZE BANK EROSION.
2. STEPS SHALL BE TAKEN UPON COMPLETION OF THIS PROJECT, TO ENSURE BANK STABILITY. THIS MAY INCLUDE, BUT NOT LIMITED TO, THE PLACEMENT OF RIPRAP OR BANK SEEDING.
3. MATERIALS USED FOR BANK PROTECTION SHALL BE EROSION RESISTANT AND FREE FROM TOXIC OR OTHER CONTAMINANTS. BROKEN ASPHALT IS SPECIFICALLY EXCLUDED FROM USE AS BANK PROTECTION.
4. CARE MUST BE EMPLOYED THROUGHOUT THE COURSE OF THIS PROJECT TO AVOID THE CREATION OF UNNECESSARY TURBIDITY WHICH MAY DEGRADE WATER QUALITY OR ADVERSELY AFFECT AQUATIC LIFE OUTSIDE OF THE PROJECT AREA.
5. IN AREAS WHERE TREES HAVE BEEN REMOVED TO FACILITATE CONSTRUCTION, THEY SHALL BE REPLACED WITH NATIVE SPECIES POSSESSING RAPID GROWTH CHARACTERISTICS.
6. THE APPLICANT SHALL PLANT NATIVE TREE SPECIES WITHIN THE RIGHT-OF-WAY ADJACENT TO RATTLESNAKE CREEK AND SUGAR CREEK TO REPLACE ANY TREES REMOVED AS A RESULT OF CONSTRUCTION ACTIVITIES.

CALCULATED DSW
CHECKED DSW

GENERAL NOTES

FAY-35-2.57

12b
522

i:\users\dwise\fay\35\257&75\401newsht\new012b.dgn 20-MAR-2001 3:34PM dwise

SHEET CREATED 1-31-01 DSW

TEMPORARY SOIL EROSION AND SEDIMENT CONTROL

THE FOLLOWING ESTIMATED QUANTITIES ARE TO BE USED AS DIRECTED BY THE ENGINEER FOR TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES:

877, TEMPORARY SEEDING AND MULCHING	177,197 SQ.YD.
877, SEDIMENT REMOVAL	16,500 CU.YD.
877, TEMPORARY SLOPE DRAINS	2,585 LIN.FT.
877, TEMPORARY PERIMETER, DITCH CHECK OR INLET PROTECTION FILTER FABRIC FENCE	11,250 CU.YD.
877, TEMPORARY SEDIMENT BASINS AND DAMS	66,000 CU.YD.
877, TEMPORARY DIKES	10,340 LIN.FT.
877, TEMPORARY DITCH PROTECTION	15,000 SQ. YD.
601, ROCK CHANNEL PROTECTION, TYPE C (WITH FILTER)	120 CU.YD.
601, ROCK CHANNEL PROTECTION, TYPE C (WITHOUT FILTER)	13,200 CU.YD.
870, COMMERCIAL FERTILIZER	38 TON
870, REPAIR SEEDING AND MULCHING	44,299 SQ.YD.
870, WATER	316 M.GAL.
870, INTER-SEEDING	41000 SQ.YD.

FARM DRAINS

ALL FARM DRAINS, WHICH ARE ENCOUNTERED DURING CONSTRUCTION, SHALL BE PROVIDED WITH UNOBSTRUCTED OUTLETS. EXISTING COLLECTORS WHICH ARE LOCATED BELOW THE ROADWAY DITCH ELEVATIONS, AND WHICH CROSS THE ROADWAY, SHALL BE REPLACED WITHIN THE (RIGHT-OF-WAY) LIMITS BY ITEM 603 CONDUIT, TYPE B, ONE COMMERCIAL SIZE LARGER THAN THE EXISTING CONDUIT.

EXISTING COLLECTORS AND ISOLATED FARM DRAINS, WHICH ARE ENCOUNTERED ABOVE THE ELEVATION OF ROADWAY DITCHES, SHALL BE OUTLETTED INTO THE ROADWAY DITCH BY 603 TYPE F CONDUIT. THE OPTIMUM OUTLET ELEVATION SHALL BE ONE FOOT ABOVE THE FLOWLINE ELEVATION OF THE DITCH. LATERAL FIELD TILES WHICH CROSS THE ROAD- WAY SHALL BE INTERCEPTED BY 603, TYPE E CONDUIT, AND CARRIED IN A LONGITUDINAL DIRECTION TO AN ADEQUATE OUTLET OR ROADWAY CROSSING.

THE LOCATION, TYPE, SIZE AND GRADE OF REPLACEMENTS SHALL BE DETERMINED BY THE ENGINEER AND PAYMENT SHALL BE MADE ON FINAL MEASUREMENTS.

EROSION CONTROL PADS AND ANIMAL GUARDS SHALL BE PROVIDED AT THE OUTLET END OF ALL FARM DRAINS AS PER STANDARD CONSTRUCTION DRAWING DM-1.1M, EXCEPT WHEN THEY OUTLET INTO A DRAINAGE STRUCTURE. PAYMENT FOR THE EROSION CONTROL PADS AND ANIMAL GUARDS AND ANY NECESSARY BENDS OR BRANCHES SHALL BE INCLUDED FOR PAYMENT IN THE PERTINENT CONDUIT ITEM.

THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED IN THE GENERAL SUMMARY FOR THE WORK NOTED ABOVE:

603, 4" CONDUIT, TYPE B	300 LIN. FT.
603, 6" CONDUIT, TYPE B	300 LIN. FT.
603, 8" CONDUIT, TYPE B	300 LIN. FT.
603, 10" CONDUIT, TYPE B	300 LIN. FT.
603, 12" CONDUIT, TYPE B	300 LIN. FT.
603, 15" CONDUIT, TYPE B	300 LIN. FT.
603, 4" CONDUIT, TYPE E	1000 LIN. FT.
603, 6" CONDUIT, TYPE E	1000 LIN. FT.
603, 8" CONDUIT, TYPE E	1000 LIN. FT.
603, 10" CONDUIT, TYPE E	500 LIN. FT.
603, 12" CONDUIT, TYPE E	500 LIN. FT.
603, 15" CONDUIT, TYPE E	500 LIN. FT.
603, 4" CONDUIT, TYPE F	300 LIN. FT.
603, 6" CONDUIT, TYPE F	300 LIN. FT.
603, 8" CONDUIT, TYPE F	300 LIN. FT.
603, 10" CONDUIT, TYPE F	300 LIN. FT.
603, 12" CONDUIT, TYPE F	300 LIN. FT.
603, 15" CONDUIT, TYPE F	300 LIN. FT.
601, ROCK CHANNEL PROTECTION TYPE C WITH FILTER	100 CU. YDS.

EROSION CONTROL

ITEMS 601, 667 AND 670 ARE PROVIDED IN THE PLANS FOR EROSION CONTROL. ROCK OF A STABLE NATURE SHALL NOT BE REMOVED IN ORDER TO PLACE ANY OF THESE ITEMS AND TURF OF A STABLE NATURE SHALL NOT BE REMOVED IN ORDER TO PLACE 667, 668 OR 670. THE ENGINEER SHALL CHECK AND NON-PERFORM QUANTITIES OR ADJUST LOCATIONS AND QUANTITIES OF THESE ITEMS WHERE INDICATED BY FIELD CONDITIONS DURING CONSTRUCTION. IN ADDITION, THESE ITEMS SHALL MEET THE REQUIREMENT OF 108.04.

ITEM 605, AGGREGATE DRAINS

AGGREGATE DRAINS SHALL BE PLACED AT FIFTY (50) FOOT INTERVALS ON EACH SIDE OF NORMAL CROWNED SECTIONS, STAGGERED SO THAT EACH DRAIN IS 25 FEET FROM THE ADJACENT DRAIN ON THE OPPOSITE SIDE, AND AT TWENTY- FIVE (25) FOOT INTERVALS ON THE LOW SIDE ONLY OF SUPERELEVATED SECTIONS. AN AGGREGATE DRAIN SHALL BE PLACED AT THE LOW POINT OF EACH SAG VERTICAL CURVE.

UNTREATED SEPTIC CONNECTIONS

THIS PLAN MAKES NO PROVISION FOR CONNECTING, NOR SHALL THE ENGINEER OR CONTRACTOR CONNECT, ANY UNTREATED SEPTIC DRAINAGE INTO THE HIGHWAY DRAINAGE SYSTEM. ANY PIPE CARRYING UNTREATED SEPTIC FLOW SHALL BE PLUGGED WITH CLASS C CONCRETE AT THE RIGHT-OF-WAY LINE. PAYMENT FOR PLUGGING SHALL BE INCLUDED IN THE CONTRACT PRICE FOR (ITEM 203 EXCAVATION) (THE PERTINENT 202 ITEM).

ITEM 407 - TACK COAT AND ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE

THE RATE OF APPLICATION OF THE 407 TACK COAT SHALL BE SUBJECT TO ADJUSTMENT AS DIRECTED BY THE ENGINEER. FOR ESTIMATING PURPOSES ONLY, THE PLAN QUANTITIES INDICATE AN AVERAGE APPLICATION RATE OF:

407, TACK COAT	0.075 GAL./SQ.YD
407, TACK COAT FOR INTERMEDIATE COURSE	0.010 GAL./SQ.YD

ITEM 611-REINFORCED CONCRETE APPROACH SLABS (t-), AS PER PLAN

TWO SEPARATE THICKNESSES OF CLEAR OR OPAQUE POLYETHYLENE FILM, 705.6, SHALL BE PLACED ON THE PREPARED SUBBASE WHERE THE APPROACH SLAB IS TO BE CONSTRUCTED. THE POLYETHYLENE FILMS SHALL COMPLETELY COVER THE FULL LENGTH AND WIDTH OF THE SUBBASE BETWEEN THE SIDEWALL FORMS AND THE APPROACH SLAB. THE COST OF THE PARAPETS IS TO BE INCLUDED WITH ITEM 611 REINFORCED CONCRETE APPROACH SLAB (t-), AS PER PLAN. MATERIAL, LABOR AND INSTALLATION FOR THE ABOVE SHALL BE INCLUDED FOR PAYMENT IN THIS ITEM 611 REINFORCED CONCRETE APPROACH SLAB (t-), AS PER PLAN.

CONVERSION OF METRIC STANDARD DRAWINGS

THE METRIC STANDARD DRAWINGS REFERENCED IN THIS PLAN SHALL BE CONVERTED TO ENGLISH UNITS USING THE SI (METRIC) TO ENGLISH CONVERSION FACTORS PROVIDED IN SECTION 109.011 OF THE 1997 CONSTRUCTION AND MATERIALS SPECIFICATIONS. THE APPENDIX OF ASTM E 380 SHALL BE UTILIZED FOR ANY ADDITIONAL CONVERSION FACTORS REQUIRED. CONVERSIONS SHALL BE APPROPRIATELY PRECISE AND SHALL REFLECT STANDARD INDUSTRY ENGLISH VALUES WHERE SUITABLE.

SANITARY WORK

THE FOLLOWING CONDUIT TYPES MAY BE USED: 707.42, 707.43, 707.44, 707.45, 707.45, 707.46, 707.47, 707.51. 707.52 PS46 MIN., 706.01, 707.02 OR 706.08 WITH JOINTS AS PER 706.11 OR 706.12.

THE PAY ITEM MEASURED PER LIN. FT. SHALL BE:

ITEM 603 - 4" CONDUIT, TYPE B, FOR SANITARY

ITEM SPECIAL, DRILLED WATER WELL ABANDONED

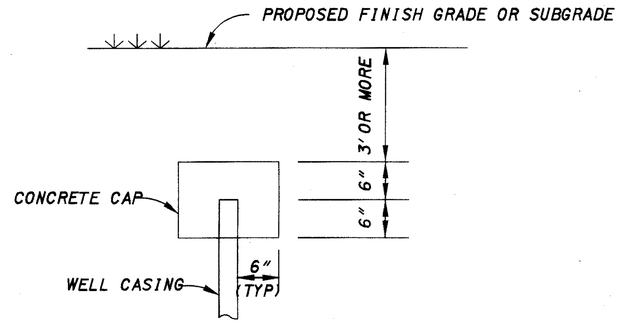
THE EXISTING CONCRETE OR STONE SLAB WELL COVER AND PUMPING EQUIPMENT SHALL BE REMOVED AND DISPOSED OF. THE CASING SHALL BE CUT OFF AT LEAST EIGHTEEN INCHES BELOW THE PROPOSED FINISHED GRADE OUTSIDE PROPOSED PAVEMENT AREA OR AT LEAST EIGHTEEN INCHES BELOW THE PROPOSED SUBGRADE ELEVATION INSIDE PROPOSED PAVEMENT AREAS. THE WELL SHALL BE FILLED FROM BOTTOM TO TOP WITH BENTONITE PELLETS OR CONCRETE MEETING ASTM C 150 TYPE I PORTLAND CEMENT WITH NO AIR ENTRAINMENT, AND THEN CAPPED. SEE DETAIL BELOW.

THE CONTRACTOR SHALL FILE WELL LOG AND ABANDONMENT FORMS WITH THE OHIO DEPARTMENT OF NATURAL RESOURCES (ODNR) AS REQUIRED BY THE OHIO REVISED CODE. ODNR REQUIRES THREE (3) WORKING DAYS NOTICE FROM THE CONTRACTOR FOR INSPECTION OF THE ABOVE MENTIONED WORK. THE PROPOSED WORK SHALL BE APPROVED BY ODNR IN WRITING PRIOR TO COMMENCEMENT. ANY ADDITIONAL MATERIALS REQUIRED BY ODNR SHALL BE CONSIDERED INCIDENTAL. ODNR'S ADDRESS IS AS FOLLOWS:

OHIO DEPARTMENT OF NATURAL RESOURCES
DIVISION OF WATER
1939 FOUNTAIN SQUARE, BUILDING E
COLUMBUS, OHIO 43224
TELEPHONE: (614) 265-6739

A COUNTY ISSUED PLUGGING PERMIT MAY ALSO BE REQUIRED.

THE CONTRACT PRICE PER EACH ITEM SPECIAL, DRILLED WATER WELL ABANDONED, SHALL INCLUDE PAYMENT FOR ALL LABOR, TOOLS, MATERIALS AND INCIDENTALS NECESSARY TO COMPLETE THIS ITEM.



NOTE: THE CONTRACTOR MAY WELD A CAP ON THE CASING OR OTHER APPROVED METHOD IN LIEU OF USING THE CONCRETE CAP AS SHOWN.

ITEM 880 - ASPHALT CONCRETE PAVEMENT WITH WARRANTY (7 YEAR)

LIFT THICKNESSES AND STEP WIDTHS SHOWN IN THE PLAN ARE FOR QUANTITY ESTIMATION ONLY, AND ARE NOT REQUIRED LIFT THICKNESSES FOR ACTUAL CONSTRUCTION.

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 1/00 BY FYC

GENERAL NOTES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\013.dgn 01-FEB-2000 2:54PM fcheek
I:\pr\project\fay35\fay-35-2.57\ 013.dgn

MAINTENANCE OF TRAFFIC SUB-SUMMARY

ITEM	SHEET NUMBERS																ITEM	ITEM EXTENSION	GRAND TOTAL	UNIT	DESCRIPTION
							15		17	18	19	20	21								
301									15								301	46000	15	CU YD	BITUMINOUS AGGREGATE BASE, PG64-22
407									2420								407	10000	2420	GALLON	TACK COAT
410							500										410	12000	500	CU YD	TRAFFIC COMPACTED SURFACE, TYPE A OR B
410							100										410	13000	100	CU YD	TRAFFIC COMPACTED SURFACE, TYPE C
448									1020								448	47020	1020	CU YD	ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
614							LUMP										614	11000	LUMP		MAINTAINING TRAFFIC
614							60										614	11100	60	HOUR	LAW ENFORCEMENT OFFICER WITH PATROL CAR
SPECIAL							40										SPECIAL	61412500	40	SQ FT	REPLACEMENT SIGN
SPECIAL							25										SPECIAL	61412600	25	EACH	REPLACEMENT DRUM
614							40		40								614	13000	80	CU YD	BITUMINOUS CONCRETE FOR MAINTAINING TRAFFIC
614										145							614	13300	145	EACH	BARRIER REFLECTOR, TYPE B
614										145							614	13350	145	EACH	OBJECT MARKER
614											0.04	0.62					614	21100	0.66	MILE	TEMPORARY CENTER LINE, CLASS I, 642 PAINT
614											0.06	0.07					614	21200	0.13	MILE	TEMPORARY CENTER LINE, CLASS I, 740.06, TYPE 1
614											0.08	1.23					614	22100	1.31	MILE	TEMPORARY EDGE LINE, CLASS I, 642 PAINT
614										2.12	0.19	0.14	0.34				614	22200	2.79	MILE	TEMPORARY EDGE LINE, CLASS I, 740.06, TYPE 1
614										5620	200						614	23400	5820	LIN FT	TEMPORARY CHANNELIZING LINE, CLASS I, 740.06, TYPE 1
615							LUMP										615	10000	LUMP		TEMPORARY ROAD
615							9934										615	25000	9934	SQ YD	TEMPORARY PAVEMENT, CLASS B
616							210										616	10000	210	M GAL	WATER
616							30										616	20000	30	TON	CALCIUM CHLORIDE
617									170								617	10100	170	CU YD	COMPACTED AGGREGATE, TYPE A
617									17								617	25000	17	M GAL	WATER
622										3560			160				622	40020	3720	LIN FT	PORTABLE CONCRETE BARRIER, 32"
642									4								642	00290	4	MILE	CENTER LINE

CALCULATED

CHECKED

MAINTENANCE OF TRAFFIC SUB-SUMMARY

FAY-35-2.57

GENERAL

THE CONTRACTOR SHALL MAINTAIN TWO WAY TRAFFIC ON ALL EXISTING ROADS AFFECTED BY THE CONTRACT EXCEPT THAT C.R. 16, TR 94, SR 729, AND C.R. 85 MAY BE CLOSED TO TRAFFIC AS NOTED BELOW IN THE VARIOUS CONSTRUCTION PHASES. SEE SHEET 17 FOR S.R. 729 DETOUR ROUTE AND DESIGNATED LOCAL DETOUR ROUTES.

CONSTRUCTION PHASING

PHASE 1: STA 277+00 TO END OF PROJECT STA 396+20.24

TWO-WAY TRAFFIC SHALL BE MAINTAINED ON HARMONY ROAD, AT ALL TIMES, EXCEPT FOR A PERIOD NOT TO EXCEED 30 CONSECUTIVE CALENDAR DAYS. THROUGH TRAFFIC SHALL BE DETOURED AS SHOWN ON SHEET 17A, TO FACILITATE THE CONSTRUCTION OF A CULVERT, AND ACCOMPANYING ROADWAY IMPROVEMENTS, FROM STA. 24+50 TO STA. 30+44. LOCAL ACCESS TO PROPERTIES SHALL BE PROVIDED. LIQUIDATED DAMAGES, PER CMS 108.07, WILL BE ASSESSED FOR CLOSURES EXCEEDING THE SPECIFIED LIMIT.

CONSTRUCT MAINLINE GRADING INCLUDING BRIDGE OVER C.R. 85. C.R. 85 SHALL BE KEPT OPEN AT ALL TIMES EXCEPT FOR A PERIOD NOT TO EXCEED FIVE (5) CALENDAR DAYS, THE ROAD MAY BE CLOSED TO TRAFFIC IN ORDER TO CONSTRUCT THE TWO CULVERTS CROSSING C.R. 85. DURING THIS PERIOD, S.R. 729 MUST BE OPENED TO TRAFFIC TO PROVIDE A LOCAL DETOUR. IN ADDITION, TRAFFIC MAY BE STOPPED FOR SHORT PERIODS OF TIME USING FLAGGERS TO ERECT THE BEAMS FOR THE BRIDGE OVER C.R. 85. WHEN THE PAVEMENT REPLACEMENT FOR THE CULVERTS CROSSING C.R. 85 HAS BEEN PLACED AND THE ROAD OPENED TO TRAFFIC, THE CONTRACTOR MAY PROCEED TO PHASE 2.

PHASE 2: STA 241+00 TO STA 277+00

CLOSE S.R. 729. CONSTRUCT APPROACHES AND BRIDGE ON S.R. 729. THE CLOSURE OF S.R. 729 SHALL NOT EXCEED 180 CALENDAR DAYS. DURING THE PERIOD S.R. 729 IS CLOSED, C.R. 85 MUST BE KEPT OPEN AT ALL TIMES AS C.R. 85 IS THE DESIGNATED LOCAL DETOUR FOR S.R. 729. LOCAL ACCESS SHALL BE MAINTAINED AT ALL TIMES ON S.R. 729.

PHASE 3: STA 224+00 TO STA 241+00

CONSTRUCT TEMPORARY ROAD TYPE B NORTH OF EXISTING USR 35 AS SHOWN ON SHEETS 23 TO 26. CONSTRUCT SERVICE ROAD 1 FROM STA 10+50 TO STA 13+00 INCLUDING DRIVES. CONSTRUCT TEMPORARY DRIVE TO SERVICE ROAD 1 USING 410 TRAFFIC COMPACTED SURFACE, TYPE A OR B TO MAINTAIN LOCAL ACCESS. MOVE TRAFFIC TO TEMPORARY ROAD AND CLOSE EXISTING USR 35.

CONSTRUCT APPROACHES, RAMPS, AND BRIDGE. COMPLETE SERVICE ROAD 1. WHEN EXISTING USR 35 IS COMPLETE, OPEN THE ROAD TO TRAFFIC. RAMPS C AND D SHALL REMAIN CLOSED UNTIL ALL WORK TO THE EAST IS COMPLETE. SEE SHEET 21. REMOVE THE TEMPORARY ROAD. CONSTRUCT SERVICE ROADS 2 AND 3.

PHASE 4: BEGIN PROJECT STA 135+72.80 TO STA 224+00

CLOSE C.R. 16 AND T.R. 94. THESE ROADS MAY BE CLOSED FOR A PERIOD NOT TO EXCEED 180 CALENDAR DAYS. THE CLOSURE MAY NOT BE PUT INTO EFFECT IF C.R. 95 IS CLOSED FOR ITS RECONSTRUCTION ON THE ADJOINING PROJECT TO THE WEST.

CONSTRUCT APPROACHES ON C.R. 16 AND T.R. 94 INCLUDING BRIDGE OVER RELOCATED USR 35. CONSTRUCT MAINLINE GRADING FROM RATTLESNAKE CREEK TO I-71 AND FROM I-71 TO STA 224+00. WORK AREAS ALONG I-71 SHALL BE PROTECTED WITH PORTABLE CONCRETE BARRIERS PLACED ON THE OUTSIDE EDGES OF THE OUTSIDE SHOULDERS DURING CONSTRUCTION OF THE APPROACHES TO I-71. SEE SHEET 18.

CONSTRUCT PIER FOR BRIDGE OVER I-71. DURING THIS PERIOD, TWO LANE TRAFFIC IN EACH DIRECTION SHALL BE MAINTAINED ON I-71 USING THE EXISTING PAVEMENT AND SHOULDERS. WORK AREA ALONG THE I-71 MEDIAN SHALL BE PROTECTED WITH PORTABLE CONCRETE BARRIERS. SEE SHEET 18.

CONSTRUCT BRIDGE OVER I-71. DURING THIS PERIOD, TRAFFIC MAY BE STOPPED FOR SHORT PERIODS OF TIME NOT TO EXCEED 15 MINUTES FOR THE PURPOSE OF ERECTING THE BEAMS. STOPPAGE OF TRAFFIC SHALL BE ACCOMPLISHED BY USE OF UNIFORMED LAW ENFORCEMENT OFFICERS WITH PATROL CARS.

PHASE 4 IS INDEPENDENT FROM THE OTHER THREE PHASES AND MAY BE CONSTRUCTED AT ANY TIME DEPENDING ON THE RESTRICTIONS NOTED ABOVE.

IF THE ADJOINING PROJECT TO THE WEST IS NOT OPEN TO TRAFFIC WHEN THIS PROJECT IS FINISHED, THE FREEWAY WILL BE CLOSED AT THE EXISTING U.S.R. 35 INTERCHANGE. SEE SHEET 21.

MAINTAINING TRAFFIC

ALL EXISTING LANES SHALL BE OPEN TO TRAFFIC BETWEEN NOVEMBER 15 AND APRIL 15. NOVEMBER 15 SHALL BE CONSIDERED TO CONSTITUTE AN INTERIM COMPLETION DATE AND LIQUIDATED DAMAGES SHALL BE ASSESSED IN ACCORDANCE WITH 108.07 FOR EACH CALENDAR DAY THAT ALL LANES ARE NOT OPEN AND AVAILABLE TO TRAFFIC.

LENGTH AND DURATION OF LANE CLOSURES AND RESTRICTIONS SHALL BE AT THE APPROVAL OF THE ENGINEER. IT IS THE INTENT TO MINIMIZE THE IMPACT TO THE TRAVELING PUBLIC. LANE CLOSURES OR RESTRICTIONS OVER SEGMENTS OF THE PROJECT IN WHICH NO WORK IS ANTICIPATED WITHIN A REASONABLE TIME FRAME, AS DETERMINED BY THE ENGINEER, SHALL NOT BE PERMITTED. THE LEVEL OF UTILIZATION OF MAINTENANCE OF TRAFFIC DEVICES SHALL BE COMMENSURATE WITH THE WORK IN PROGRESS.

THE CONTRACTOR SHALL PROVIDE, ERECT AND MAINTAIN STANDARD 48" X30" "ROAD CLOSED" SIGNS, SIGN SUPPORTS, BARRICADES, GATES AND LIGHTS, AS DETAILED IN STANDARD CONSTRUCTION DRAWING MT-101.60 AT THE FOLLOWING LOCATIONS DURING PERIODS IN WHICH THE AFFECTED ROADS ARE CLOSED TO TRAFFIC:

C.R. 16 JUST NORTH OF EXIST USR 35 INTERSECTION
C.R. 16 JUST SOUTH OF S.R. 734 INTERSECTION
T.R. 94 JUST SOUTH OF S.R. 734 INTERSECTION
S.R. 729 JUST NORTH OF C.R. 85 INTERSECTION
S.R. 729 JUST SOUTH OF USR 35 INTERSECTION
C.R. 85 JUST SOUTH OF EXIST USR 35 INTERSECTION
C.R. 85 JUST EAST OF T.R. 86 INTERSECTION
HARMONY RD. JUST SOUTH OF PROP. USR 35 INTERSECTION
HARMONY RD. JUST NORTH OF FORD ROAD INTERSECTION

ITEM 614 - LAW ENFORCEMENT OFFICER WITH PATROL CAR

IN ADDITION TO THE REQUIREMENTS OF 614 AND THE LATEST EDITION OF THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (OMUTCD), A UNIFORMED LAW ENFORCEMENT OFFICER AND OFFICIAL PATROL CAR WITH WORKING TOP MOUNTED EMERGENCY FLASHING LIGHTS SHALL BE PROVIDED FOR CONTROLLING TRAFFIC FOR THE FOLLOWING TASKS ON I-71 AND ON EXISTING U.S.R. 35:

FOR LANE CLOSURES: DURING INITIAL SET-UP PERIODS, TEAR DOWN PERIODS, SUBSTANTIAL SHIFTS OF A CLOSURE POINT OR WHEN NEW OR LANE CLOSURE ARRANGEMENTS ARE INITIATED.

DURING THE ENTIRE ADVANCE PREPARATION AND CLOSURE SEQUENCE WHERE COMPLETE BLOCKAGE OF TRAFFIC IS REQUIRED.

LAW ENFORCEMENT OFFICERS (L.E.O.'S) SHOULD NOT BE USED WHERE THE OMUTCD INTENDS THAT FLAGGERS BE USED. THE LEO'S ARE CONSIDERED TO BE EMPLOYED BY THE CONTRACTOR AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR THEIR ACTIONS. ALTHOUGH THEY ARE EMPLOYED BY THE CONTRACTOR, THE PROJECT ENGINEER SHALL HAVE CONTROL OVER THEIR PLACEMENT. THE OFFICIAL PATROL CAR SHALL BE A PUBLIC SAFETY VEHICLE AS REQUIRED BY THE OHIO REVISED CODE.

THE CONTRACTOR SHALL MAKE ARRANGEMENTS FOR THESE SERVICES WITH: FAYETTE COUNTY SHERIFF'S OFFICE.

LAW ENFORCEMENT OFFICERS WITH PATROL CAR REQUIRED BY THE TRAFFIC MAINTENANCE TASKS ABOVE SHALL BE PAID FOR ON A UNIT PRICE HOURLY BASIS UNDER ITEM 614 - LAW ENFORCEMENT OFFICER (WITH PATROL CAR). THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN CARRIED TO THE GENERAL SUMMARY.

ITEM 614: LAW ENFORCEMENT OFFICER W/PATROL CAR - 60 HRS.

THE HOURS PAID SHALL INCLUDE MINIMUM SHOW-UP TIME REQUIRED BY THE LAW ENFORCEMENT AGENCY INVOLVED.

IF THE CONTRACTOR WISHES TO UTILIZE LEO'S FOR FLAGGING AND TRAFFIC CONTROL OTHER THAN FOR THAT REQUIRED IN THESE PLANS, HE MAY DO SO AT HIS OWN EXPENSE. PAYMENT FOR THE EXCESS ABOVE THE CONTRACT REQUIREMENTS WILL BE INCLUDED UNDER ITEM 614 MAINTAINING TRAFFIC.

DUST CONTROL

THE CONTRACTOR SHALL FURNISH AND APPLY WATER AND CALCIUM CHLORIDE FOR DUST CONTROL AS DIRECTED BY THE ENGINEER. THE FOLLOWING CONTINGENCY QUANTITIES HAVE BEEN INCLUDED FOR DUST CONTROL PURPOSES:

616, WATER	200 M. GAL.
616, CALCIUM CHLORIDE	20 TONS

ITEM 622, PORTABLE CONCRETE BARRIER

IT IS ANTICIPATED THAT THE SAME BARRIER WILL BE USED IN VARIOUS PHASES OF CONSTRUCTION. MOVEMENT OF THE CONCRETE BARRIER BETWEEN PHASES SHALL BE ACCOMPLISHED IN ONE WORKING DAY. FLAGGERS SHALL BE UTILIZED FOR PROTECTION OF VEHICULAR TRAFFIC UNTIL MOVEMENT OF THE BARRIER IS COMPLETE.

ALL COSTS INVOLVED IN REMOVING AND REINSTALLING THE CONCRETE BARRIER WILL BE INCLUDED IN THE CONTRACT PRICE FOR ITEM 622, PORTABLE CONCRETE BARRIER.

ITEM 614, BARRIER REFLECTORS AND OBJECT MARKERS

BARRIER REFLECTORS AND/OR OBJECT MARKERS SHALL BE INSTALLED ON ALL PORTABLE CONCRETE BARRIER USED FOR MAINTAINING TRAFFIC. BARRIER REFLECTORS SHALL CONFORM TO ITEM 626, EXCEPT THAT THE SPACING SHALL BE 25 FEET. OBJECT MARKERS 9"X15" WITH ORANGE REFLECTIVE SHEETING, TYPE G (730.19), SHALL BE MOUNTED ON THE TOP SURFACE OF THE PCB MIDWAY BETWEEN THE FACE MOUNTED BARRIER REFLECTORS ALSO ON THE TOP SURFACE OF EACH TAPERED END SECTION AND EXPOSED ENDS. AN ESTIMATED QUANTITY OF ITEM 614 - BARRIER REFLECTORS, TYPE B AND ITEM 614 - OBJECT MARKERS HAVE BEEN PROVIDED IN THE MAINTENANCE OF TRAFFIC PLANS AND CARRIED TO THE GENERAL SUMMARY.

ITEM SPECIAL, REPLACEMENT SIGN

FLAT SHEET SIGNS FURNISHED BY THE CONTRACTOR IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS, SPECIFICATIONS AND PROPOSAL WHICH BECOME DAMAGED BY TRAFFIC FOR REASONS BEYOND THE CONTROL OF THE CONTRACTOR SHALL BE REPLACED IN KIND WHEN ORDERED BY THE ENGINEER. REPLACEMENT SIGNS SHALL BE NEW. OTHER MATERIALS MAY BE USED, BUT IN GOOD CONDITION, SUBJECT TO APPROVAL BY THE ENGINEER.

PAYMENT FOR THE NEW SIGNS SHALL BE MADE AT THE CONTRACT PRICE PER SQUARE FEET FOR ITEM SPECIAL, REPLACEMENT SIGN, AN SHALL INCLUDE THE COST OF REMOVING AND DISPOSING OF DAMAGED SIGNS, HARDWARE AND SUPPORTS, AND PROVIDING THE NECESSARY REPLACEMENT HARDWARE, SUPPORTS, ETC.

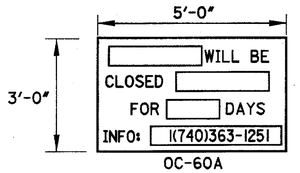
AN ESTIMATED QUANTITY OF 40 SQUARE FEET HAS BEEN PROVIDED IN THE GENERAL SUMMARY.

COVERING OF SIGNS

WHERE THE PLANS CALL FOR A PERMANENT SIGN TO BE COVERED, THE CONTRACTOR SHALL DO SO IN SUCH A MANNER AS TO AVOID DAMAGING THE PERMANENT SIGN WHEN THE COVER IS REMOVED. THE COVER SHALL BE TOTALLY OPAQUE. THE USE OF ADHESIVE TAPE APPLIED DIRECTLY TO A SIGN FACE IS STRICTLY PROHIBITED.

NOTICE OF CLOSURE SIGNS

NOTICE OF CLOSURE SIGNS, AS DETAILED IN THESE PLANS, SHALL BE ERECTED BY THE CONTRACTOR AT LEAST ONE WEEK IN ADVANCE OF THE SCHEDULED ROAD CLOSURE. THE SIGNS SHALL BE ERECTED ON THE RIGHT HAND SIDE OF THE ROAD FACING TRAFFIC. THEY SHALL BE PLACED SO AS NOT TO INTERFERE WITH THE VISIBILITY OF ANY OTHER TRAFFIC CONTROL SIGNS. ON ROADWAYS, THEY SHOULD BE ERECTED AT THE POINT OF CLOSURE.



OC-60A

CONSTRUCTION INITIATION

THE CONTRACTOR SHALL ADVISE THE DISTRICT COMMUNICATIONS OFFICER AT 1-614-363-1251, EXTENSION 469 OR BY FAX AT 1-740-369-7437 FOURTEEN (14) DAYS PRIOR TO THE START OF CONSTRUCTION ACTIVITIES. THE CONTRACTOR WILL IMMEDIATELY INFORM THE DISTRICT MAINTENANCE OF TRAFFIC ENGINEER (EXTENSION 323) OF ANY AND ALL DELAYS AND/OR CHANGES REGARDING THE CONSTRUCTION PROJECT. THE PROJECT ENGINEER WILL PROVIDE CLARIFICATION FOR ANY QUESTIONS ABOUT THIS NOTIFICATION REQUIREMENT.

ITEM 614, MAINTAINING TRAFFIC

ALL WORK AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH 614 AND OTHER APPLICABLE PORTIONS OF THE SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR 614, MAINTAINING TRAFFIC, UNLESS SEPARATELY ITEMIZED IN THE PLAN.

THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED IN THE GENERAL SUMMARY FOR USE AS DIRECTED BY THE ENGINEER FOR THE MAINTENANCE OF TRAFFIC.

614	BITUMINOUS CONCRETE FOR MAINTAINING TRAFFIC	40 CY
410	TRAFFIC COMPACTED SURFACE, TYPE A OR B	500 CY
410	TRAFFIC COMPACTED SURFACE, TYPE C	100 CY
615	TEMPORARY ROAD	LUMP
615	TEMPORARY PAVEMENT, CLASS B	9,934 SY
616	WATER	10 MGAL
616	CALCIUM CHLORIDE	10 TON

ITEM SPECIAL, REPLACEMENT DRUM

DRUMS FURNISHED BY THE CONTRACTOR IN ACCORDANCE WITH THE REQUIREMENTS OF THE PLANS, SPECIFICATIONS AND PROPOSAL WHICH BECOME DAMAGED BY TRAFFIC FOR REASONS BEYOND THE CONTROL OF THE CONTRACTOR SHALL BE REPLACED IN KIND WHEN ORDERED BY THE ENGINEER. REPLACEMENT DRUMS SHALL BE NEW.

PAYMENT FOR THE NEW DRUMS SHALL BE MADE AT THE CONTRACT PRICE PER EACH FOR ITEM SPECIAL, REPLACEMENT DRUM, AND SHALL INCLUDE THE COST OF REMOVING AND DISPOSING OF THE DAMAGED DRUM, AND PROVIDING AND MAINTAINING THE REPLACEMENT DRUM IN ACCORDANCE WITH THE CONTRACT REQUIREMENTS FOR THE ORIGINAL DRUM.

AN ESTIMATED QUANTITY OF 25 EACH HAS BEEN PROVIDED IN THE GENERAL SUMMARY.

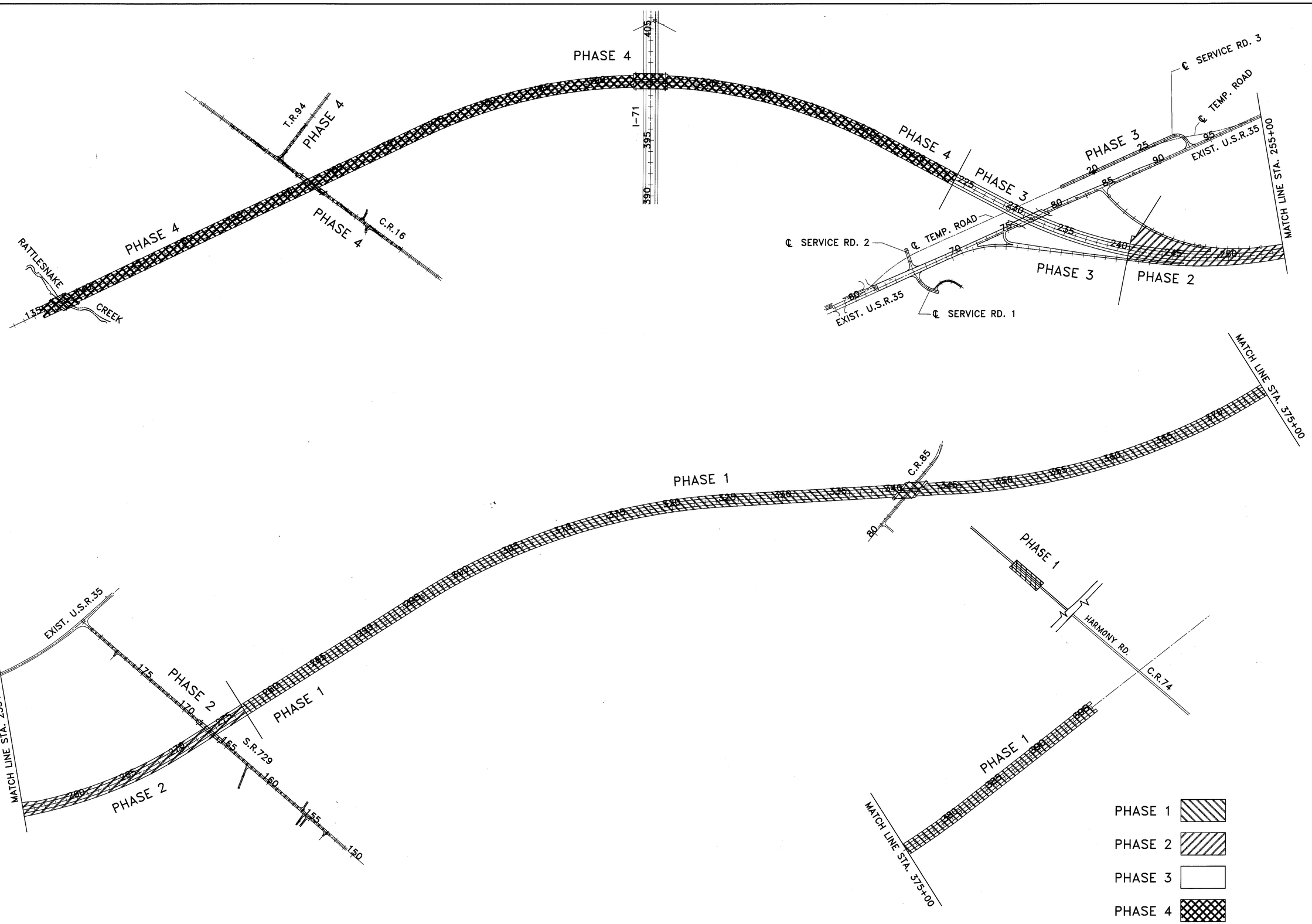
CALCULATED WGY 11-18-96
CHECKED WGY 11-18-96
REVISED 1/00 BY FYC

MAINTENANCE OF TRAFFIC NOTES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\015.dgn 01-FEB-2000 2:38PM fcheek

I:\pr\project\fay35\fay-35-2.57\ 015.dgn



REVISED 1/00 BY FYC

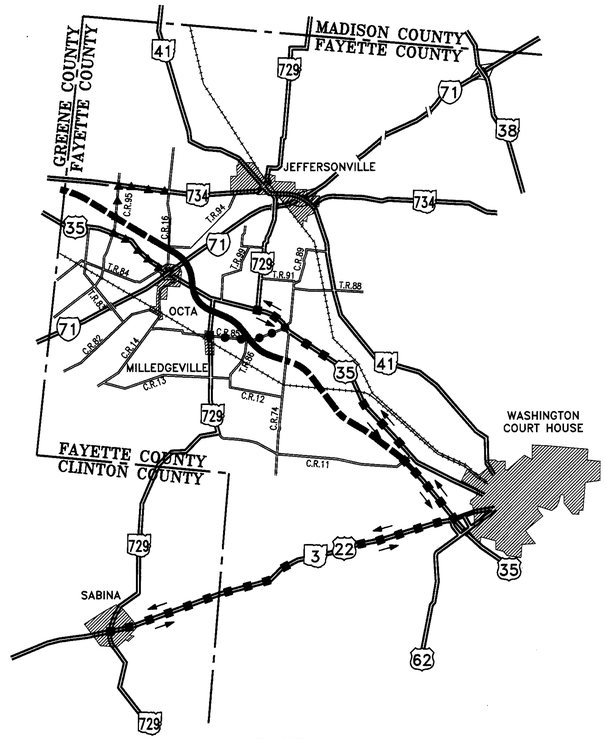
CALCULATED

CHECKED

MAINTENANCE OF TRAFFIC

FAY-35-2.57

16
522



DETOUR MAP

LEGEND

S.R. 729 DETOUR .....

S.R. 729 – DESIGNATED LOCAL DETOUR ROUTE .....

C.R. 16 – DESIGNATED LOCAL DETOUR ROUTE .....

DESIGNATED LOCAL DETOUR ROUTE

IN ADDITION TO THE OFFICIAL, SIGNED DETOUR ROUTE, LOCAL ROUTES HAVE BEEN DETERMINED TO BE THE SECONDARY, UNSIGNED DETOUR ROUTE OR "DESIGNATED LOCAL DETOUR ROUTE". THESE ROUTES ARE SHOWN ON THIS SHEET. DURING THE TIME THAT TRAFFIC IS DETOURED, THE CONTRACTOR SHALL MAINTAIN THESE ROUTES IN A CONDITION WHICH IS REASONABLY SMOOTH AND FREE FROM HOLES, RUTS, RIDGES, BUMPS, DUST AND STANDING WATER. ONCE THE DETOUR IS REMOVED AND TRAFFIC RETURNED TO ITS NORMAL PATTERN, THE DESIGNATED LOCAL DETOUR ROUTE SHALL BE RESTORED TO A CONDITION THAT IS EQUIVALENT TO THAT WHICH EXISTED PRIOR TO ITS USE FOR THIS PURPOSE. ALL SUCH WORK SHALL BE PERFORMED WHEN AND AS DIRECTED BY THE ENGINEER.

THE FOLLOWING ESTIMATED QUANTITIES ARE PROVIDED FOR USE AS DIRECTED BY THE ENGINEER TO MAINTAIN AND SUBSEQUENTLY RESTORE THE DESIGNATED LOCAL DETOUR ROUTE.

	LOCAL DETOUR ROUTE		TOTAL *
	C.R.16	S.R.729	
ITEM 301 – BITUMINOUS AGGREGATE BASE, PG64–22	5	10	15 C.Y.
ITEM 448 – ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64–22	160	860	1020 C.Y.
ITEM 614 – BITUMINOUS CONCRETE FOR MAINTENANCE OF TRAFFIC	20	20	40 C.Y.
ITEM 407 – TACK COAT	420	2000	2420 GAL.
ITEM 617 – COMPACTED AGGREGATE, TYPE A	30	140	170 C.Y.
ITEM 617 – WATER	3	14	17 M.GAL.
ITEM 642 – CENTERLINE	2	2	4 MI.

* TOTALS TO SHEET NO.

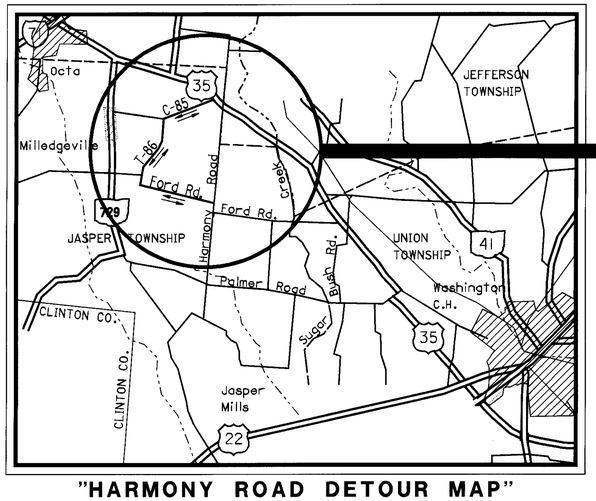
CALCULATED | CHECKED

DETOUR MAP

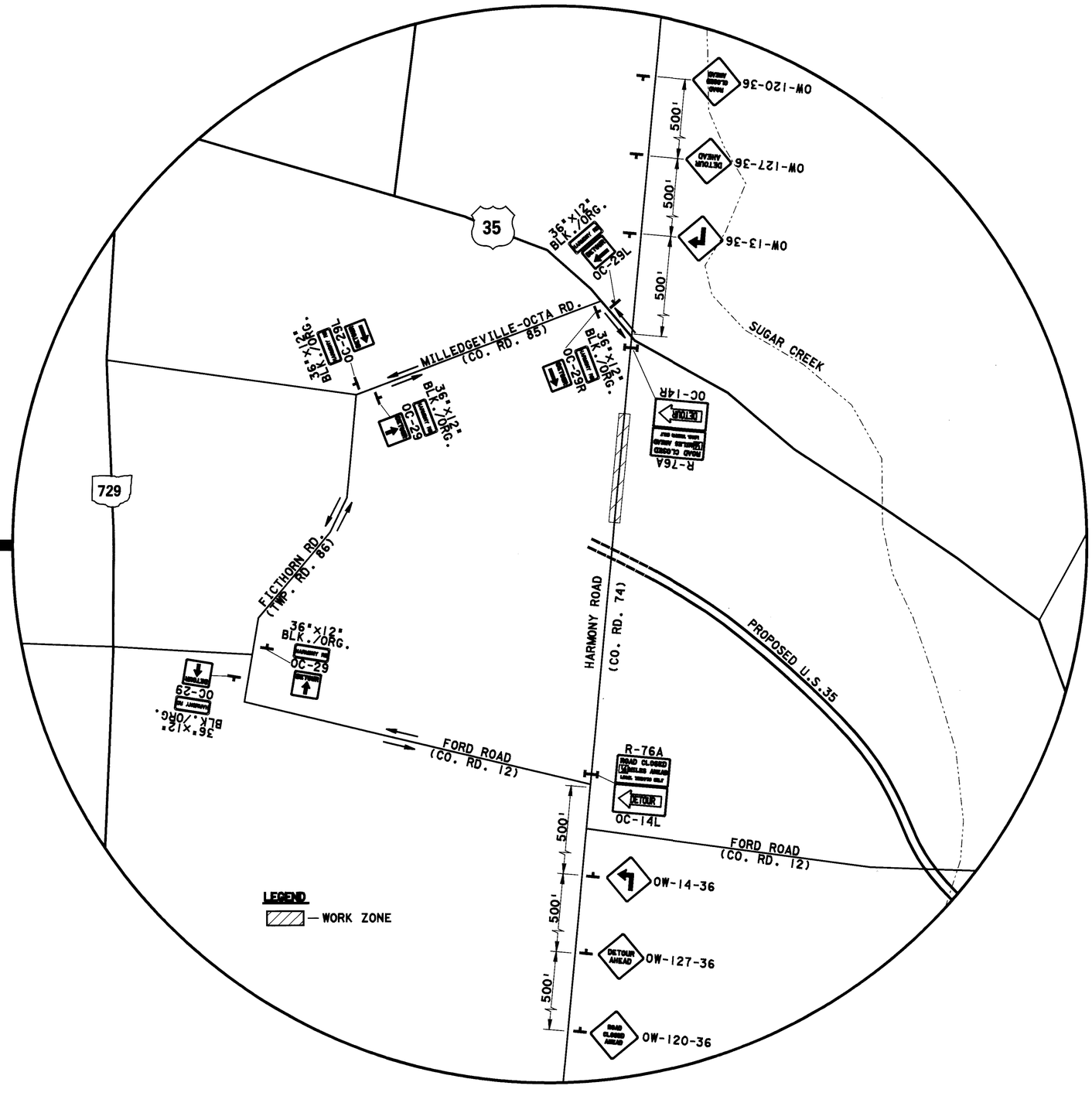
FAY–35–2.57

HARMONY ROAD (County Road 74)

HARMONY ROAD IS TO BE CLOSED TO FACILITATE CONSTRUCTION OF A CULVERT, AND ACCOMPANYING ROADWAY IMPROVEMENTS FROM STA. 24+50 TO STA. 30+44. A DETOUR ROUTE HAS BEEN PROVIDED AS SHOWN ON THIS SHEET. LOCAL ACCESS TO PROPERTIES SHALL BE PROVIDED.



"HARMONY ROAD DETOUR MAP"



"DETOUR ROUTE SIGNING"
NOT TO SCALE
(DETOUR SIGNS TO BE INSTALLED, MAINTAINED, & SUBSEQUENTLY REMOVED BY ODOT DISTRICT 6)

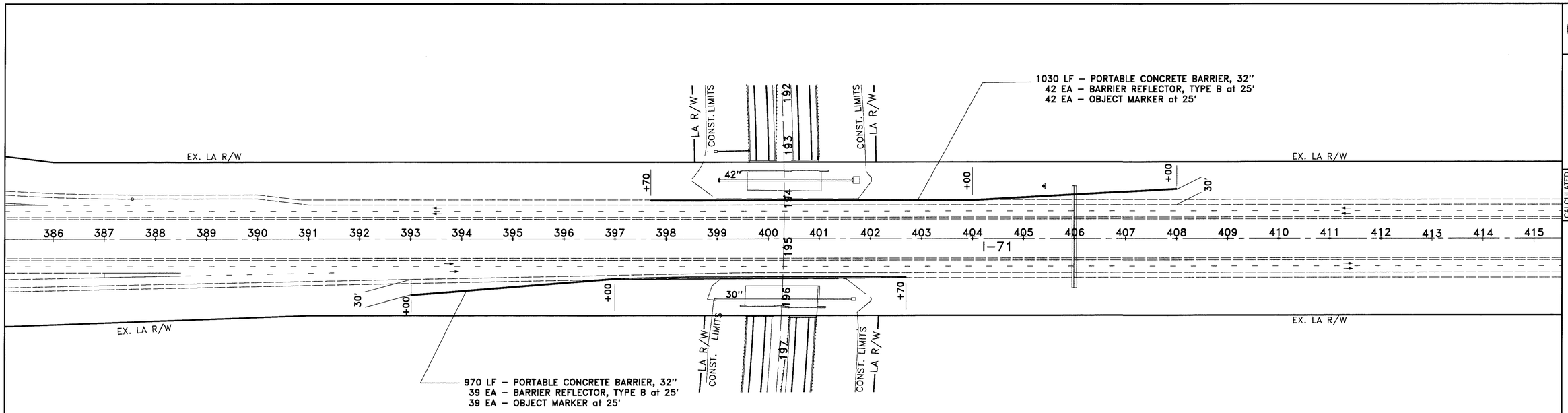
REVISED	CALCULATED
11/99	EJC
BY	CHECKED
FYC	JPC

HARMONY ROAD - M.O.T. DETOUR ROUTE

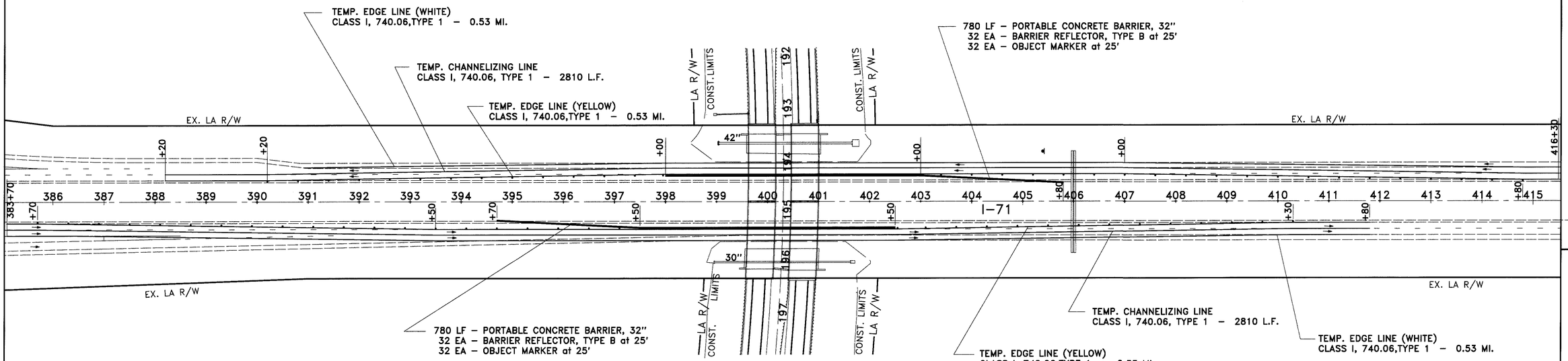
FAY-35-2.57

17A
522

cccccSYSTIMEccccc cccccPRFccccc ccccDGNcSPECIFICATIONccccc



CONSTRUCTION OF APPROACH FILL AND ABUTMENTS



CONSTRUCTION OF PIER AND BRIDGE DECK

SEE MT–102.10 FOR DETAILS

ESTIMATED QUANTITIES THIS SHEET

ITEM 622	PORTABLE CONCRETE BARRIER, 32 "	3560 L.F.
ITEM 614	BARRIER REFLECTOR, TYPE B	145 EACH
ITEM 614	OBJECT MARKER	145 EACH
ITEM 614	TEMPORARY EDGE LINE, CLASS I, 740.06, TYPE 1	2.12 MILE
ITEM 614	TEMPORARY CHANNELIZING LINE, CLASS I, 740.06, TYPE 1	5620 L.F.

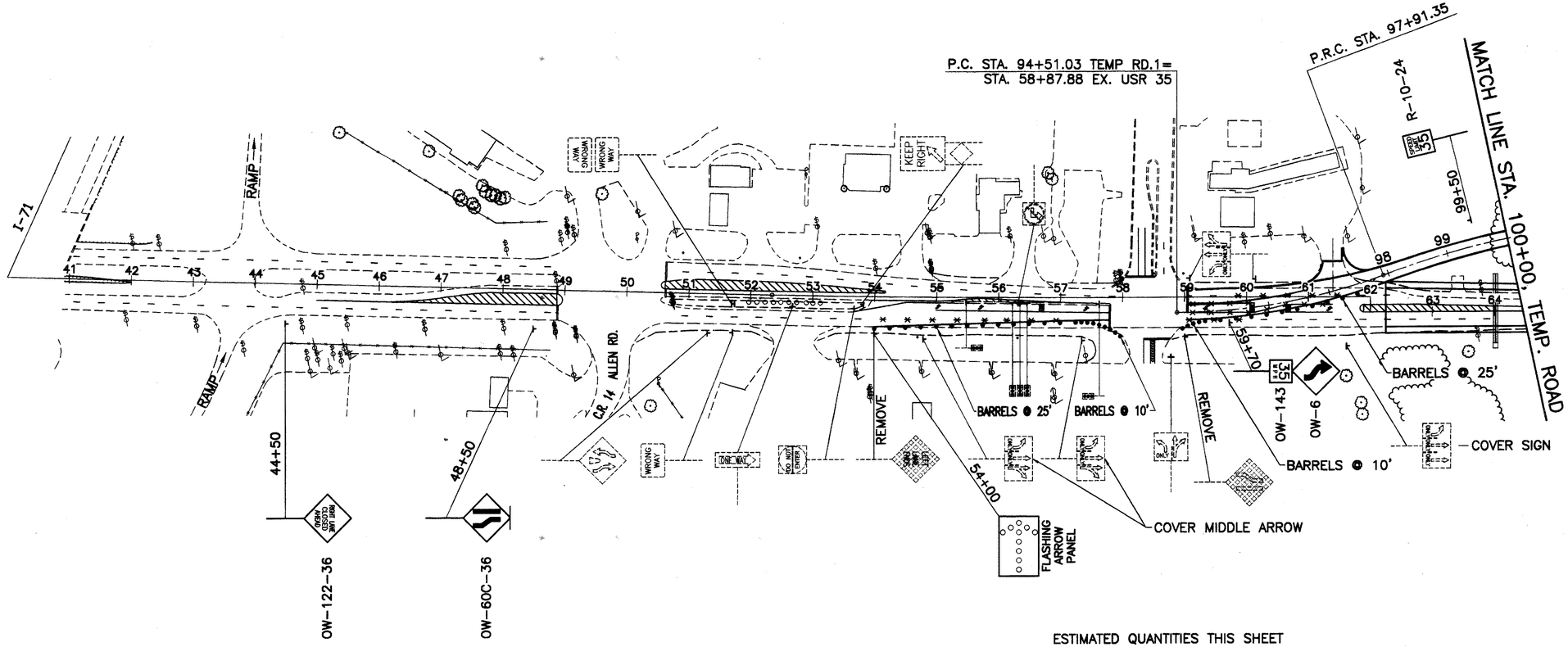
SCALE IN FEET 0 50 100 200

CALCULATED

CHECKED

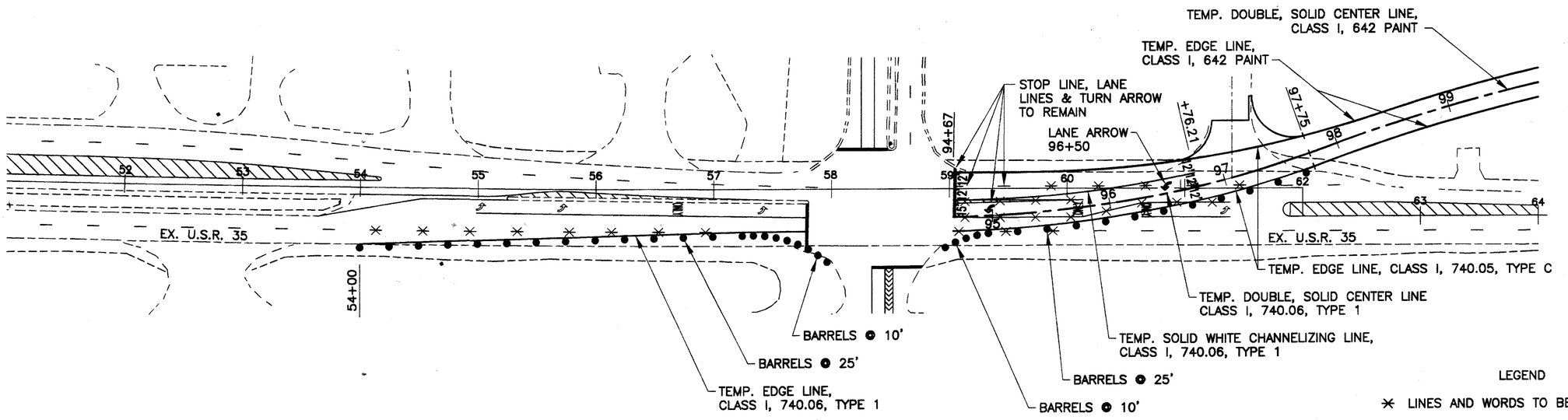
MAINTENANCE OF TRAFFIC I–71

FAY–35–2.57



ESTIMATED QUANTITIES THIS SHEET

TEMPORARY EDGE LINE, CLASS I, 642 PAINT	= 0.08 MILE
TEMPORARY EDGE LINE, CLASS I, 740.06, TYPE 1	= 0.19 MILE
TEMPORARY CENTERLINE, CLASS I, 642 PAINT	= 0.04 MILE
TEMPORARY CENTERLINE, CLASS I, 740.06, TYPE 1	= 0.06 MILE
TEMPORARY CHANNELIZING LINE, CLASS I, 740.06, TYPE 1	= 200 L.F.



LEGEND

⁂ LINES AND WORDS TO BE REMOVED.

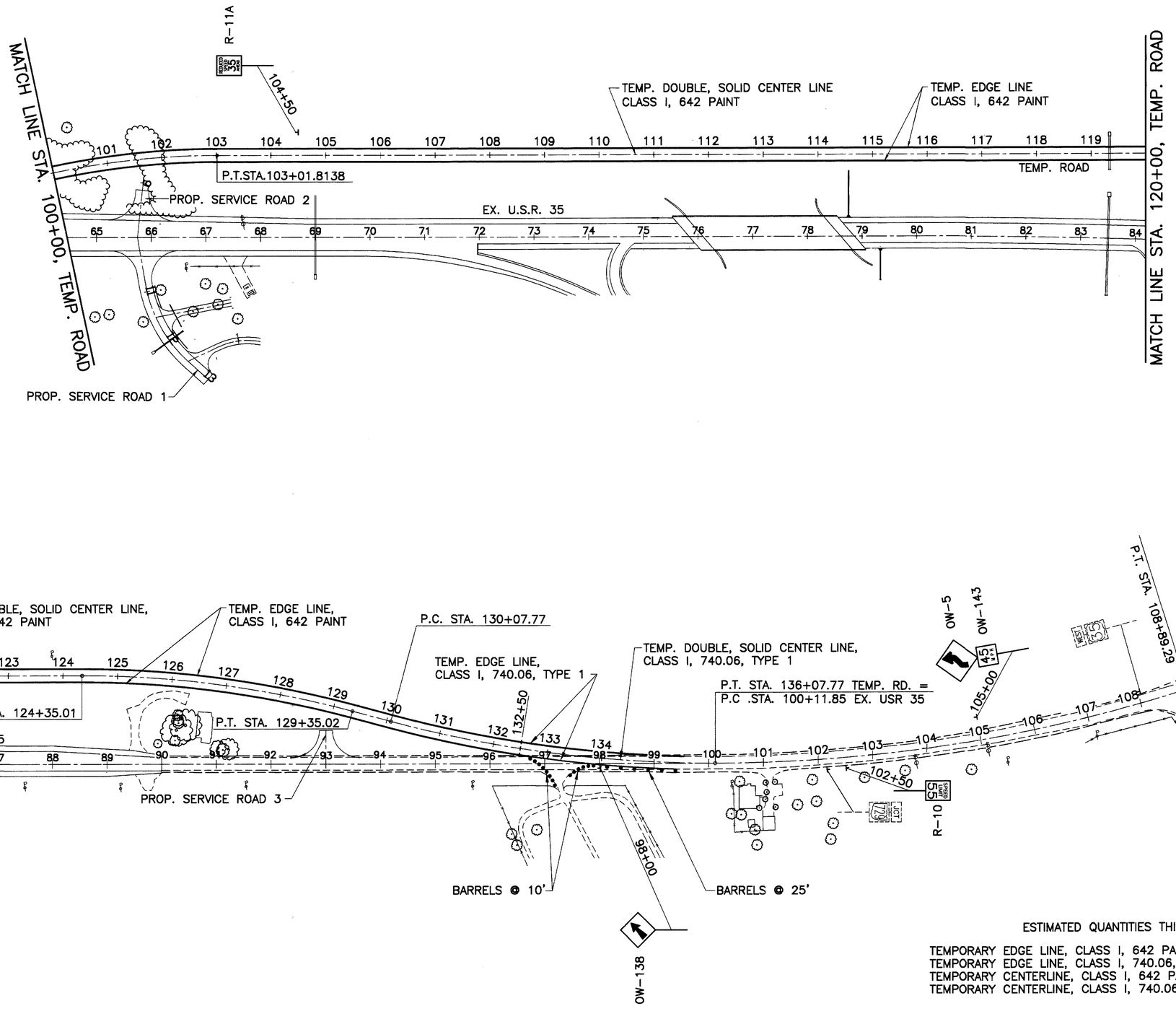
SCALE IN FEET 0 50 100 200

CALCULATED

CHECKED

MAINTENANCE OF TRAFFIC EXISTING U.S.R. 35

FAY-35-2.57



ESTIMATED QUANTITIES THIS SHEET

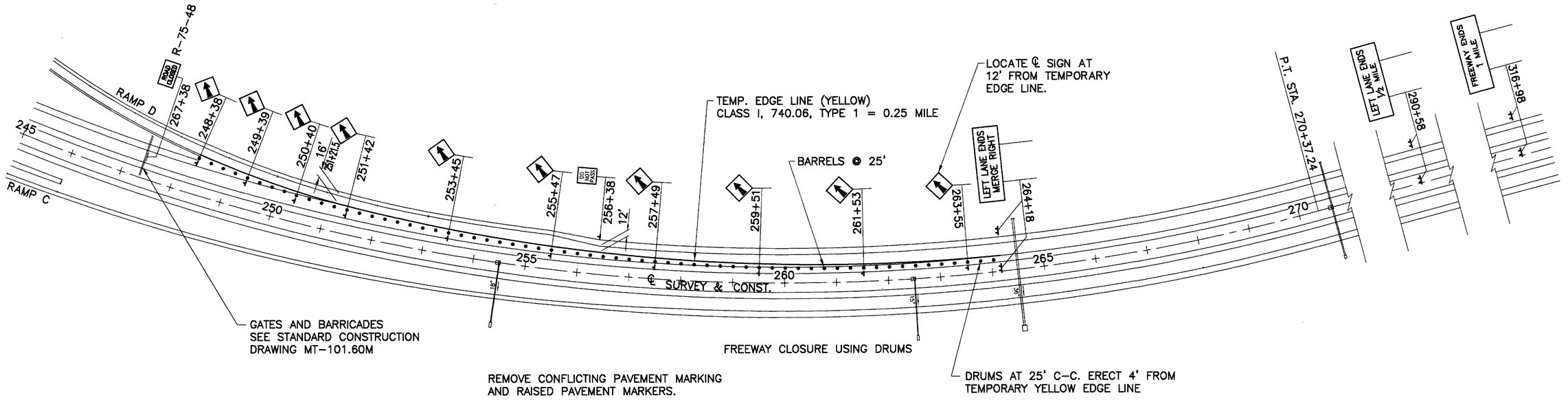
TEMPORARY EDGE LINE, CLASS I, 642 PAINT	= 1.23 MILE
TEMPORARY EDGE LINE, CLASS I, 740.06, TYPE 1	= 0.14 MILE
TEMPORARY CENTERLINE, CLASS I, 642 PAINT	= 0.62 MILE
TEMPORARY CENTERLINE, CLASS I, 740.06, TYPE 1	= 0.07 MILE

SCALE IN FEET
0 50 100 200

CALCULATED
CHECKED

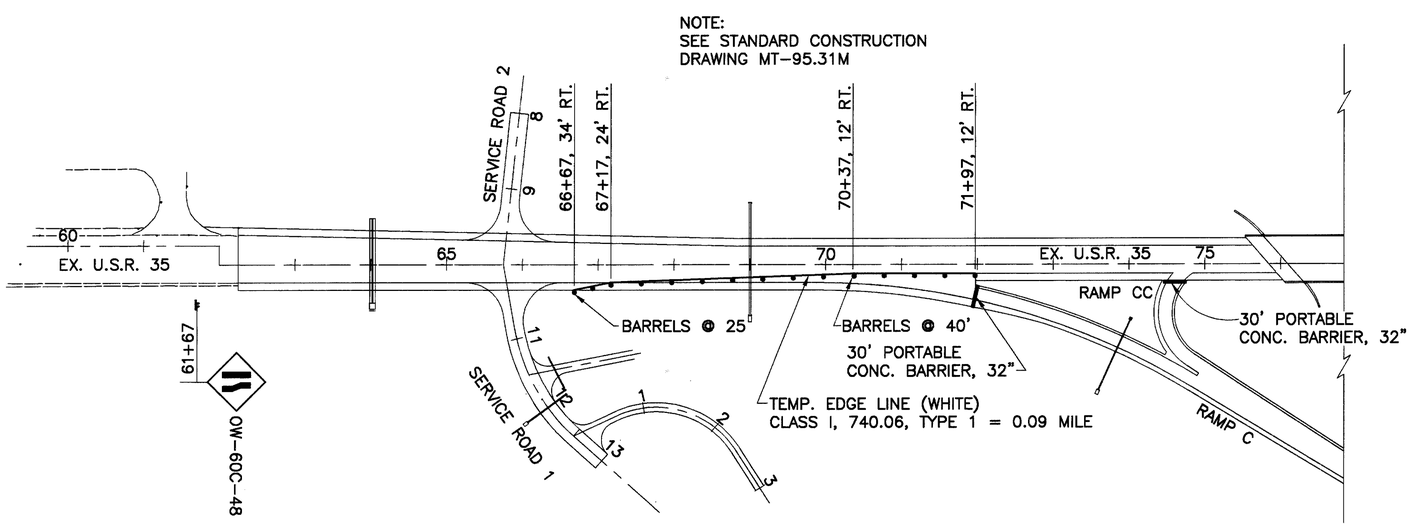
MAINTENANCE OF TRAFFIC EXISTING U.S.R. 35

FAY-35-2.57

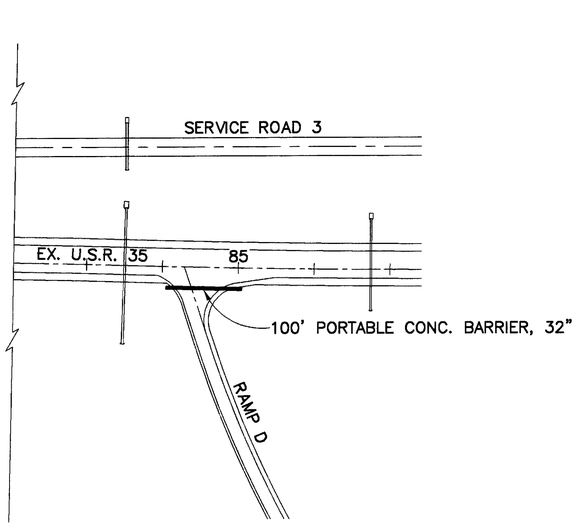


WEST BOUND CLOSURE OF PROPOSED U.S.R. 35 (IF NEEDED)
PHASE 4 SEE SHEET NO.15

ESTIMATED QUANTITIES THIS SHEET		
614	TEMPORARY EDGE LINE, CLASS I, 740.06, TYPE 1	= 0.34 MILE
622	PORTABLE CONCRETE BARRIER	= 160 L.F.



RAMP C CLOSURE



RAMP D CLOSURE

CALCULATED
CHECKED

SCALE IN FEET
0 50 100 200

MAINTENANCE OF TRAFFIC PROPOSED U.S.R. 35

FAY-35-2.57

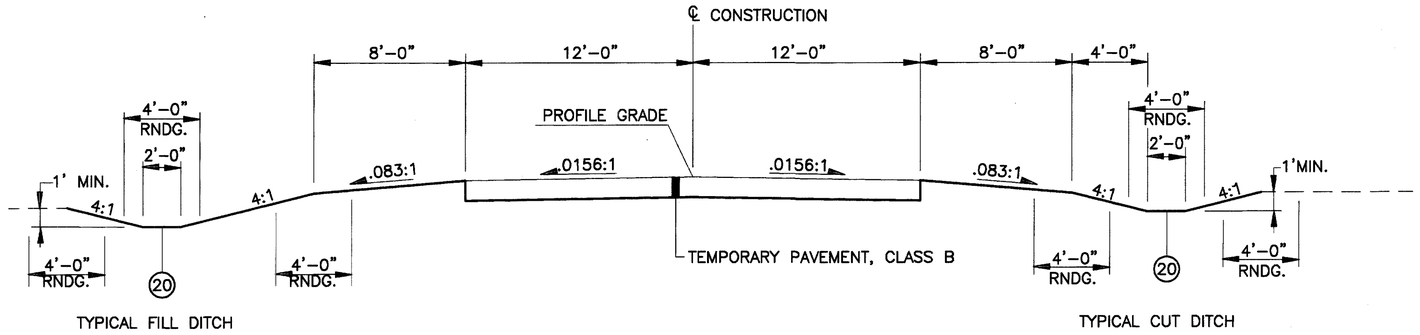
21
522

SUPERELEVATION TABLE

TEMPORARY ROAD

STATION	EDGE PAVEMENT	CENTER	EDGE PAVEMENT
97+62.41	1058.70	1059.10	MEET EXIST.
97+75	1058.67	1059.05	1059.34
98+00	1058.87	1059.03	1059.17
98+25	1059.11	1059.11	1059.11
98+41.78	1059.31	1059.23	1059.15
98+50	1059.43	1059.31	1059.19
98+75	1059.80	1059.56	1059.32
99+00	1060.15	1059.81	1059.47
	FULL SUPER @ 0.028		
102+75	1063.20	1062.86	1062.52
103+00	1063.22	1062.98	1062.74
103+01.81	1063.23	1062.99	1062.75
103+25	1063.21	1063.09	1062.90
103+50	1063.18	1063.18	1062.99
103+75	1063.21	1063.27	1063.08
104+00	1063.22	1063.34	1063.15
104+25	1063.21	1063.40	1063.21
123+25	1058.42	1058.61	1058.42
123+50	1058.40	1058.51	1058.32
123+75	1058.40	1058.41	1058.22
124+00	1058.38	1058.31	1058.12
124+25	1058.38	1058.21	1058.02
124+50	1058.36	1058.11	1057.86
	FULL SUPER @ 0.021		
129+00	1057.46	1057.21	1056.96
129+25	1057.34	1057.16	1056.98
129+50	1057.21	1057.11	1057.01
129+75	1057.08	1057.06	1057.04
130+00	1056.96	1057.01	1057.06
130+25	1056.83	1056.96	1057.09
130+50	1056.71	1056.91	1057.11
	FULL SUPER @ 0.017		
132+50	1056.11	1056.31	1056.51
133+00	1055.87	1056.07	MEET EXIST.
133+50	1055.56	1055.75	
134+00	1055.27	MEET EXIST.	
134+50	1054.75	MEET EXIST.	
135+00	1054.33	MEET EXIST.	

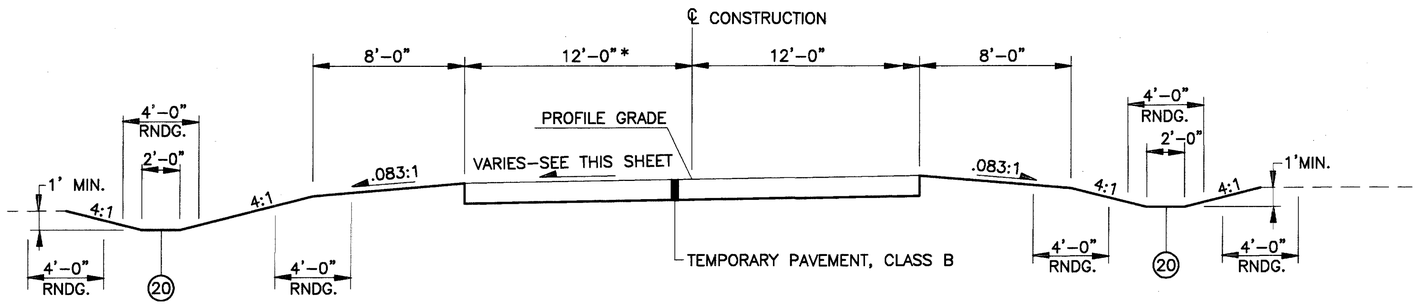
TYPICAL SECTIONS



NORMAL SECTION

STA. 104+25.00 TO STA. 123+25.00

* TAPER FROM 16.69' AT STA. 97+62.41 TO 12' AT STA. 98+41.78



SUPERELEVATED SECTION

STA. 94+51.03 TO STA. 104+25 (SHOWN OPPOSITE HAND)
STA. 123+25.00 TO STA. 136+07.77

CALCULATED J.L.G. CHECKED W.G.Y.

TYPICAL SECTIONS – TEMPORARY ROAD

FAY–35–2.57

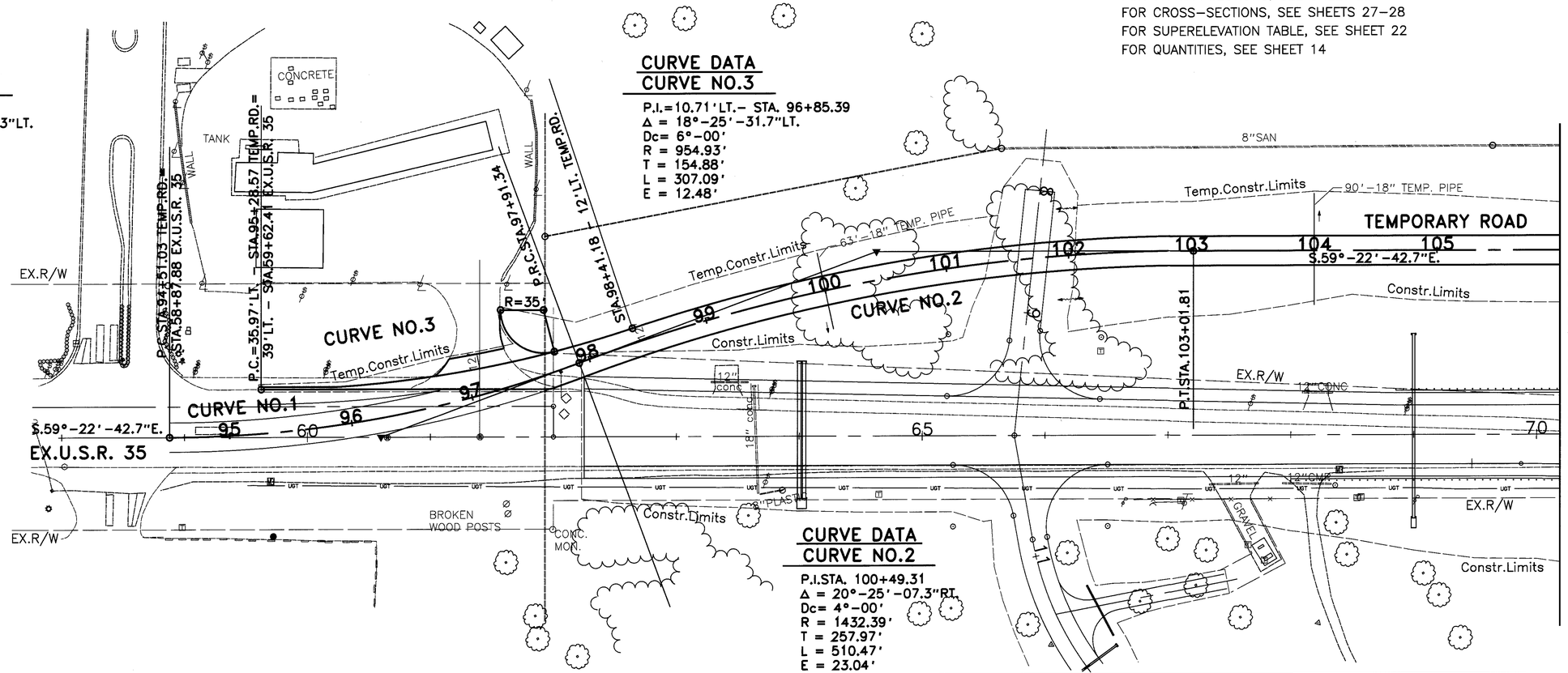
Q:\3590\3590TYP8

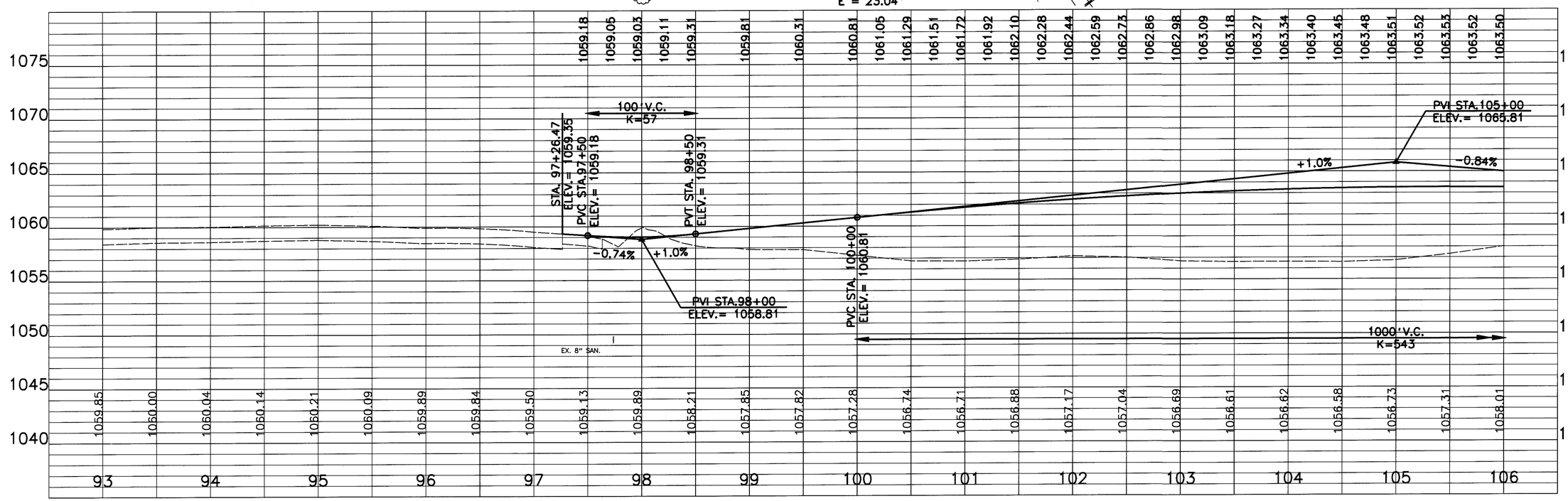
FOR TYPICAL SECTIONS, SEE SHEET 22
FOR CROSS-SECTIONS, SEE SHEETS 27-28
FOR SUPERELEVATION TABLE, SEE SHEET 22
FOR QUANTITIES, SEE SHEET 14

CURVE DATA
CURVE NO.1
P.I.STA. 96+23.01
Δ = 20°-25'-07.3"LT.
Dc= 6°-00'
R = 954.93'
T = 171.98'
L = 340.31'
E = 15.36'

CURVE DATA
CURVE NO.3
P.I.=10.71'LT.- STA. 96+85.39
Δ = 18°-25'-31.7"LT.
Dc= 6°-00'
R = 954.93'
T = 154.88'
L = 307.09'
E = 12.48'

CURVE DATA
CURVE NO.2
P.I.STA. 100+49.31
Δ = 20°-25'-07.3"RT.
Dc= 4°-00'
R = 1432.39'
T = 257.97'
L = 510.47'
E = 23.04'





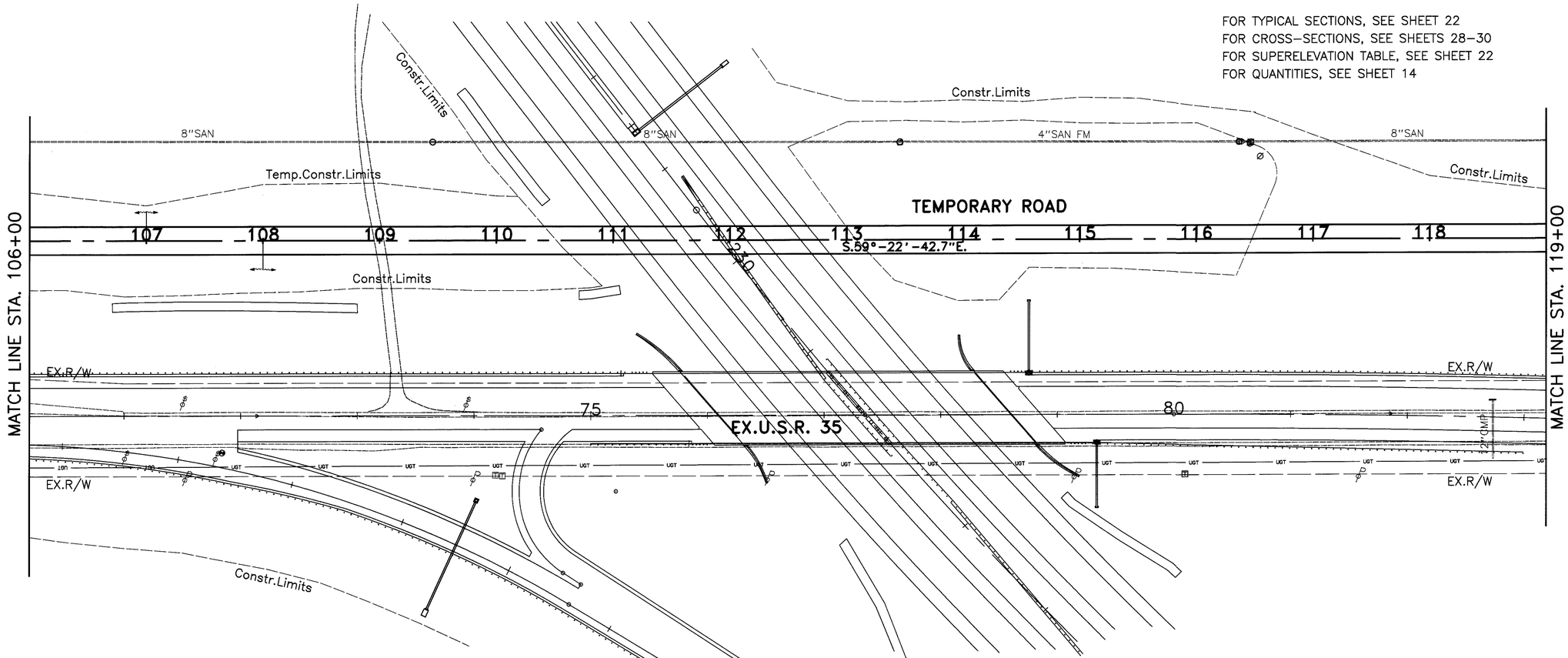
PLAN & PROFILE TEMPORARY ROAD
STA. 93+00 TO STA. 106+00

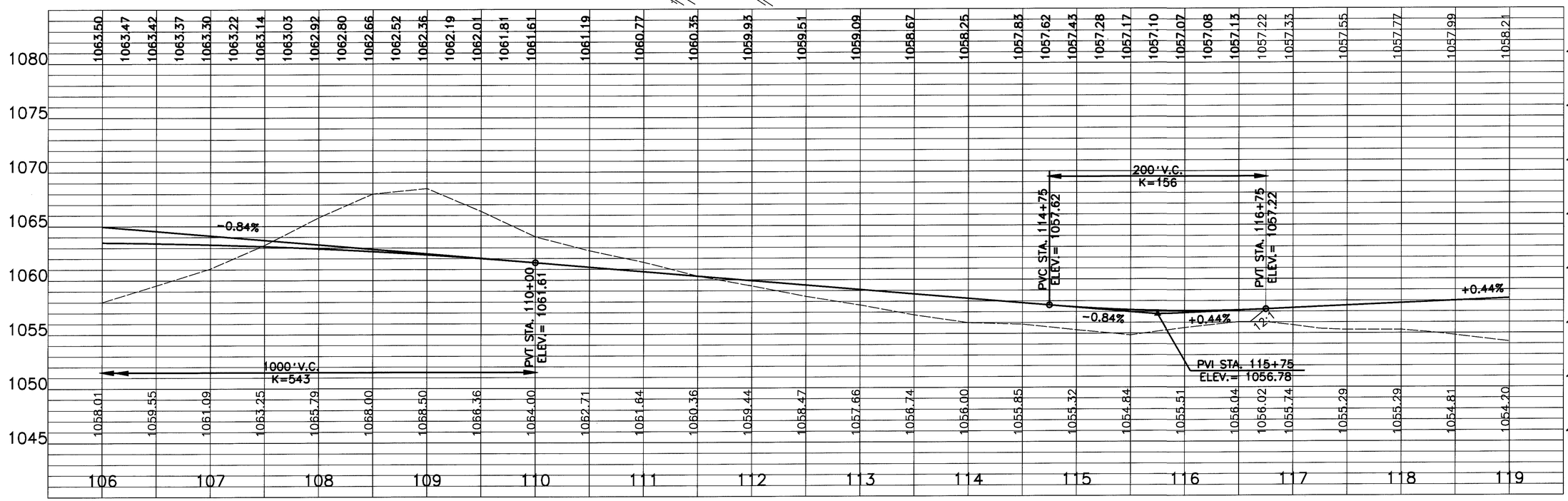
FAY-35-2.57

CALCULATED
CHECKED

HORIZONTAL SCALE IN FEET
0 25 50 100

FOR TYPICAL SECTIONS, SEE SHEET 22
FOR CROSS-SECTIONS, SEE SHEETS 28-30
FOR SUPERELEVATION TABLE, SEE SHEET 22
FOR QUANTITIES, SEE SHEET 14





0 25 50 100 HORIZONTAL SCALE IN FEET

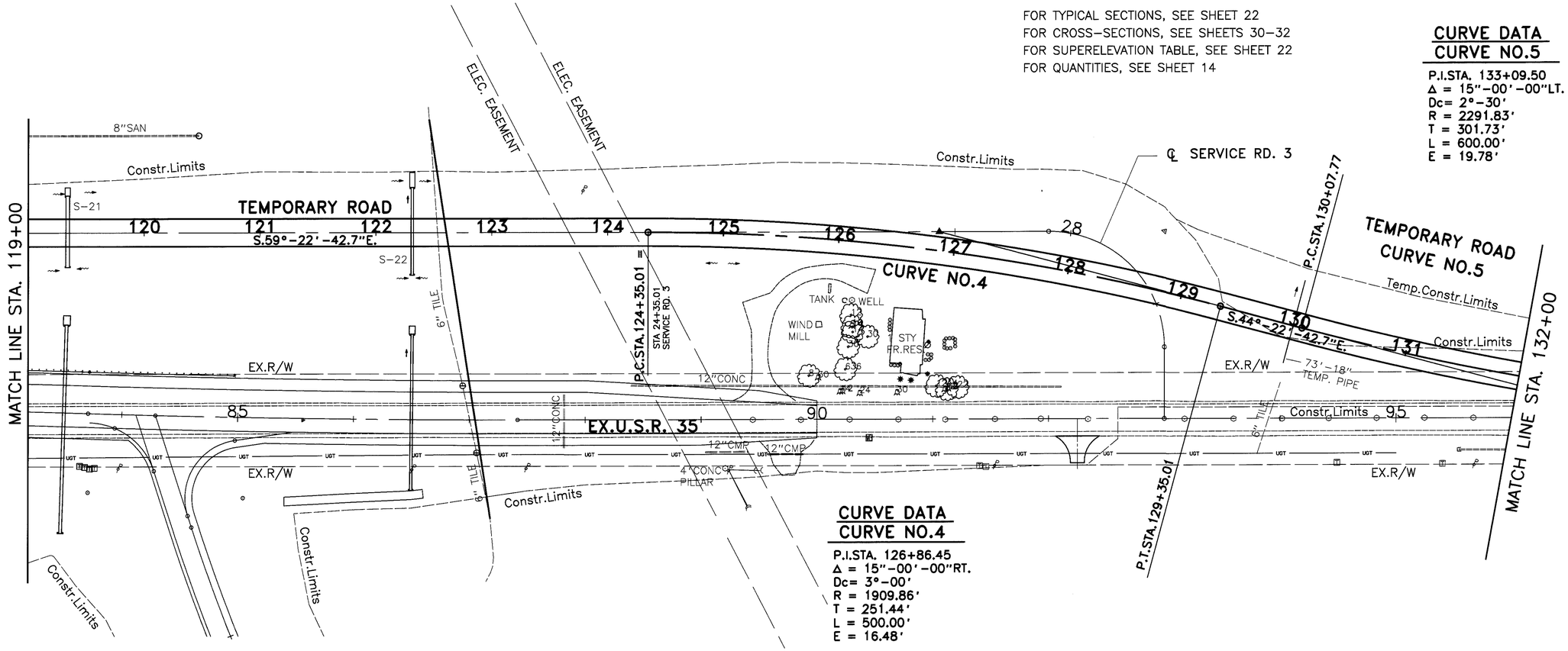
CALCULATED | CHECKED

PLAN & PROFILE TEMPORARY ROAD
STA. 106+00 TO STA. 119+00

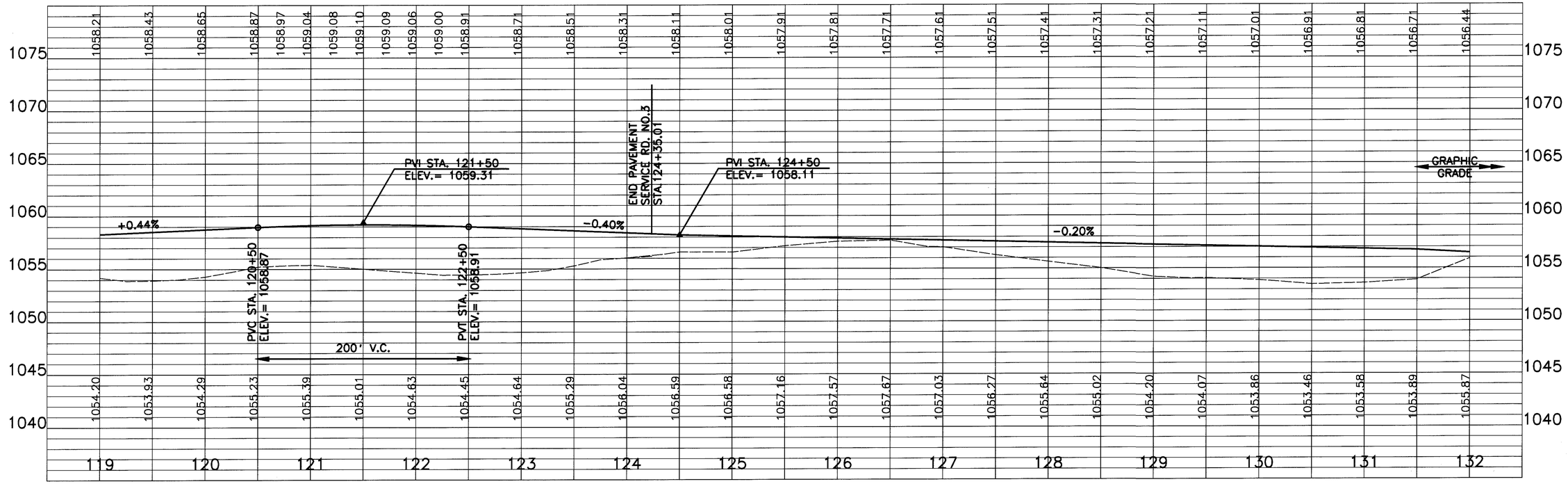
FAY-35-2.57

FOR TYPICAL SECTIONS, SEE SHEET 22
FOR CROSS-SECTIONS, SEE SHEETS 30-32
FOR SUPERELEVATION TABLE, SEE SHEET 22
FOR QUANTITIES, SEE SHEET 14

CURVE DATA
CURVE NO.5
P.I.STA. 133+09.50
Δ = 15"-00'-00"LT.
Dc= 2°-30'
R = 2291.83'
T = 301.73'
L = 600.00'
E = 19.78'



CURVE DATA
CURVE NO.4
P.I.STA. 126+86.45
Δ = 15"-00'-00"RT.
Dc= 3°-00'
R = 1909.86'
T = 251.44'
L = 500.00'
E = 16.48'



HORIZONTAL SCALE IN FEET
0 25 50 100
CALCULATED
CHECKED

PLAN & PROFILE TEMPORARY ROAD
STA. 119+00 TO STA. 132+00

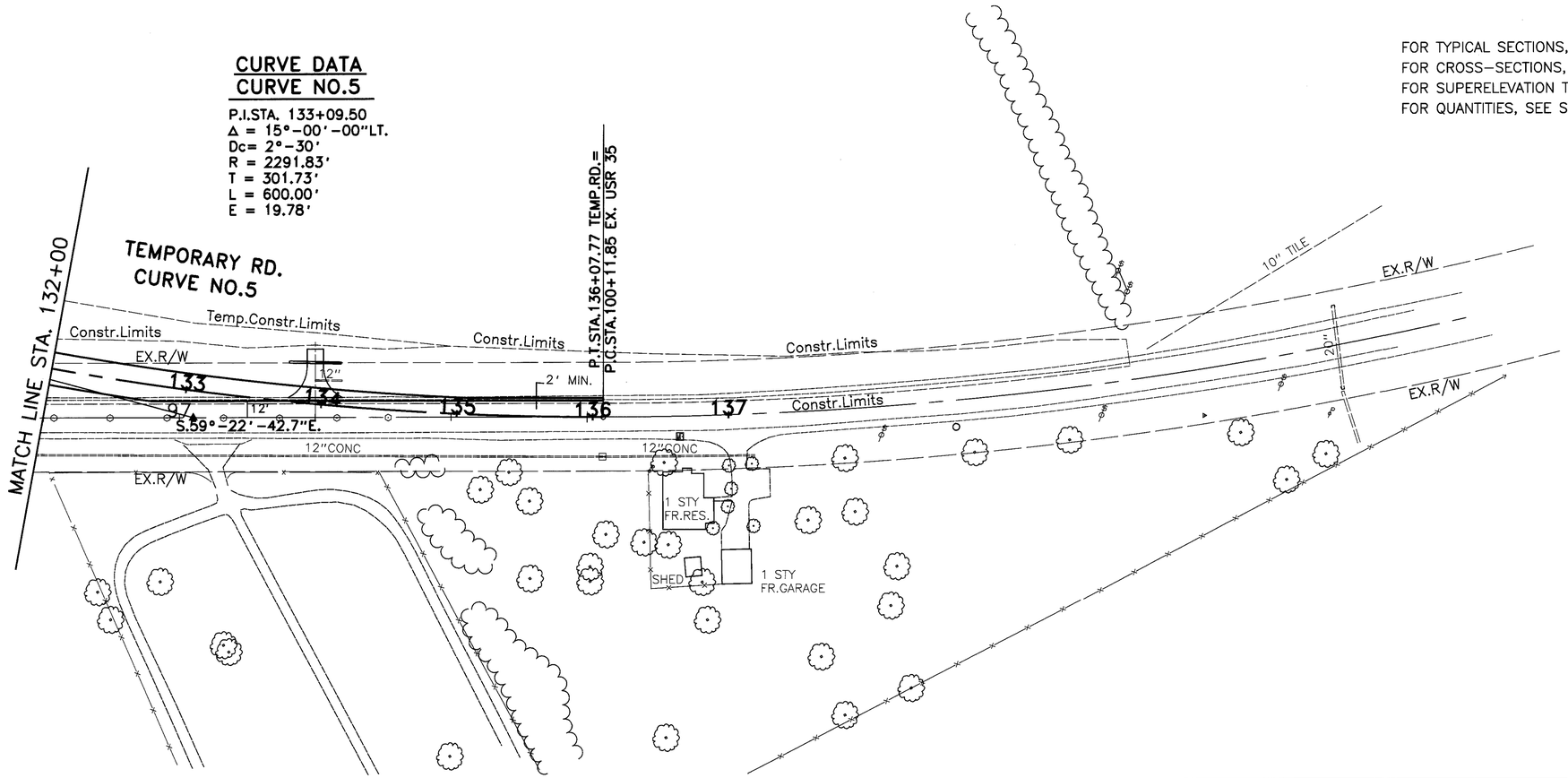
FAY-35-2.57

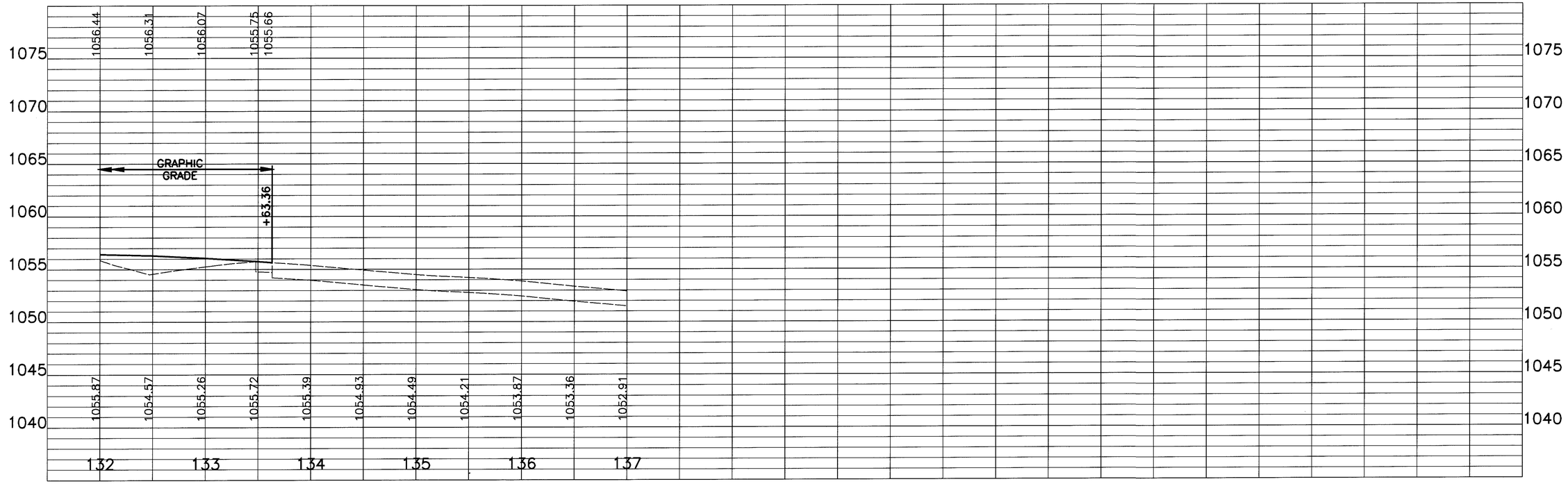
25
522

CURVE DATA
CURVE NO.5

P.I.STA. 133+09.50
Δ = 15°-00'-00"LT.
Dc= 2°-30'
R = 2291.83'
T = 301.73'
L = 600.00'
E = 19.78'

FOR TYPICAL SECTIONS, SEE SHEET 22
FOR CROSS-SECTIONS, SEE SHEET 32
FOR SUPERELEVATION TABLE, SEE SHEET 22
FOR QUANTITIES, SEE SHEET 14





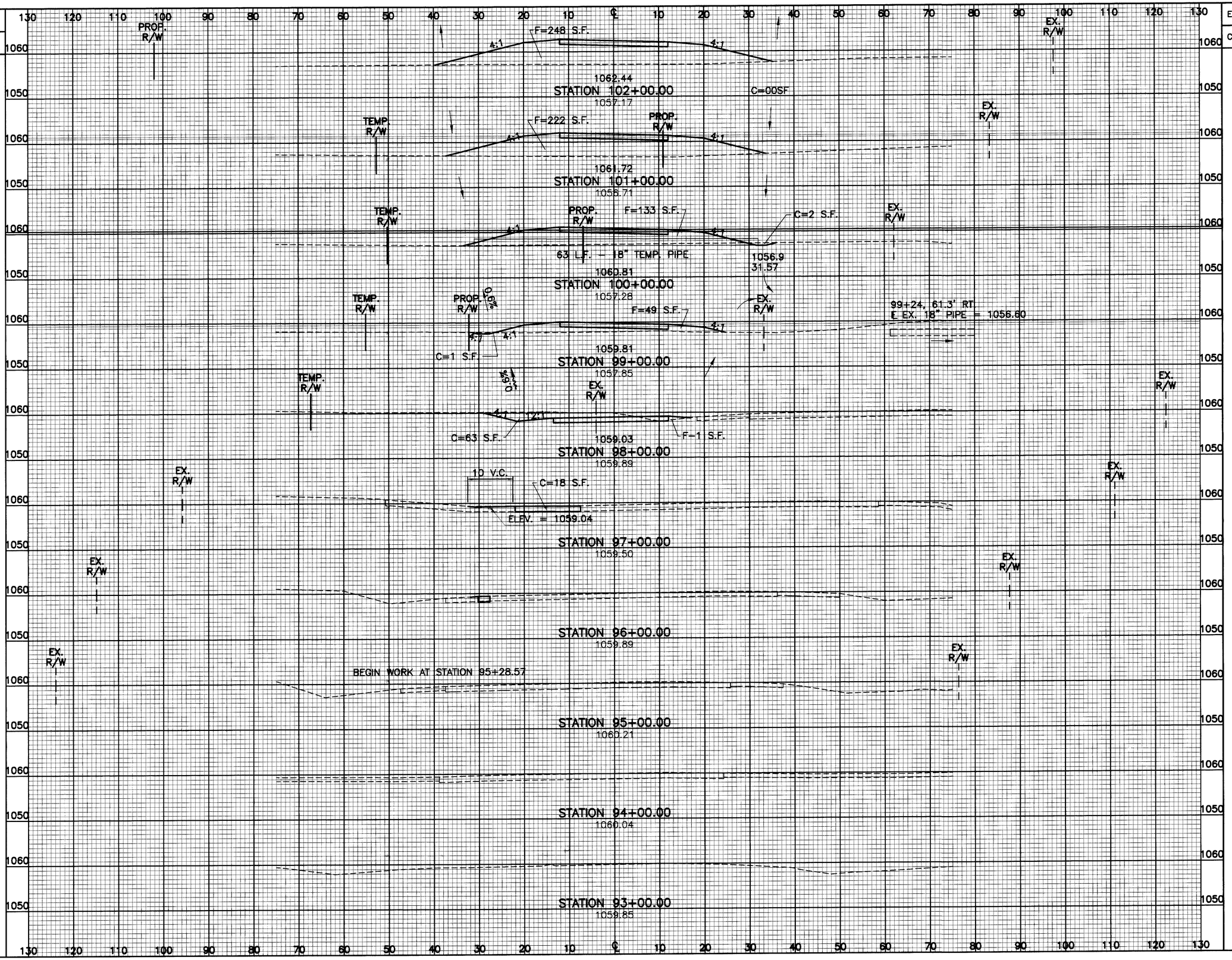
HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED | CHECKED

PLAN & PROFILE TEMPORARY ROAD
STA. 132+00 TO STA. 137+00

FAY-35-2.57

SEEDING		END AREA		VOLUME		CALCULATED J.L.C.	CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL		

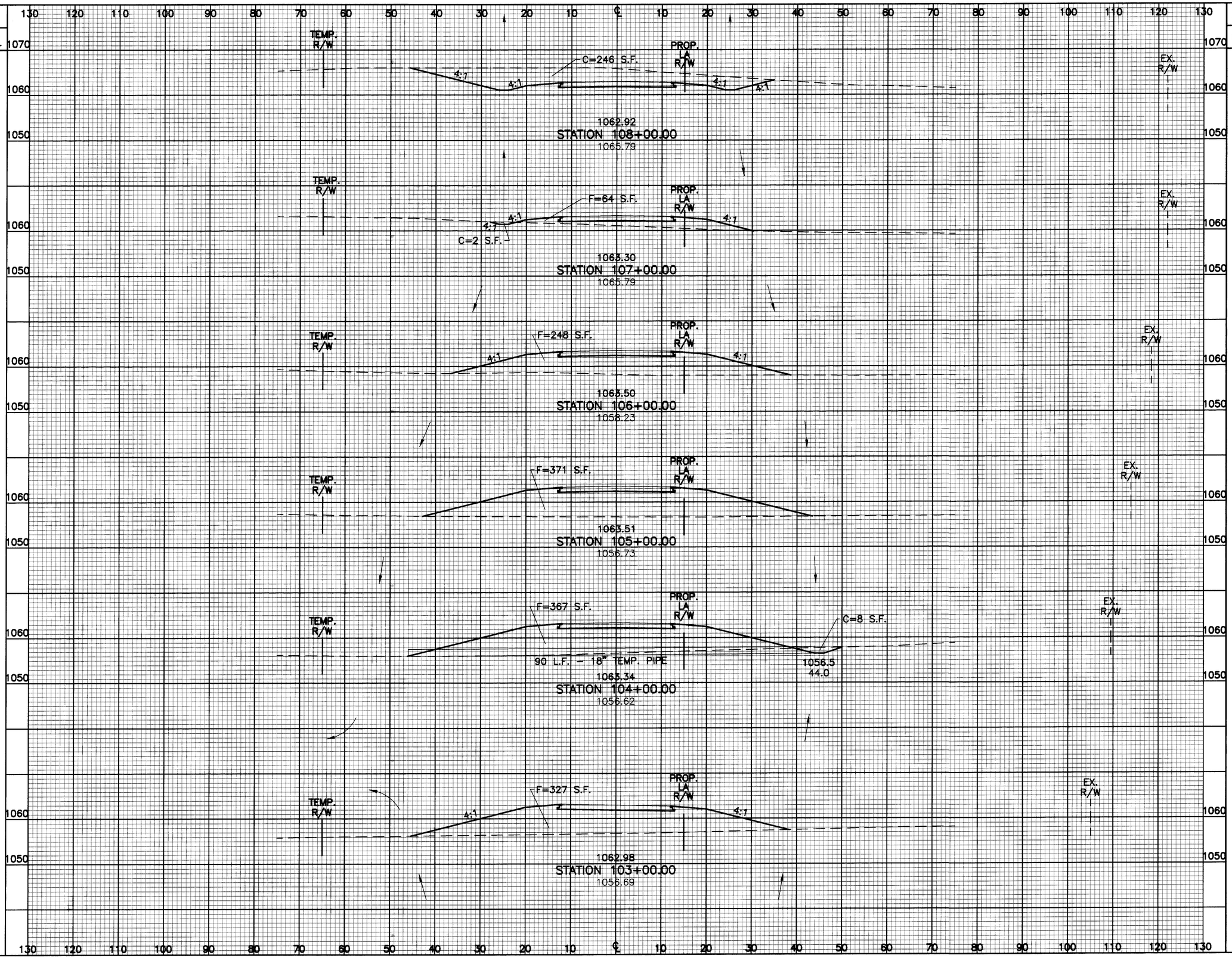


TEMPORARY ROAD X-SECTIONS STATION 93+00.00 TO STATION 102+00.00

FAY-35-2.57

3590TX1.DWG

SEEDING	
END WIDTH	SQ. YDS.



END AREA		VOLUME		CALCULATED J.L.C.	CHECKED W.G.Y.
CUT	FILL	CUT	FILL		

TEMPORARY ROAD X-SECTIONS STATION 103+00.00 TO STATION 108+00.00

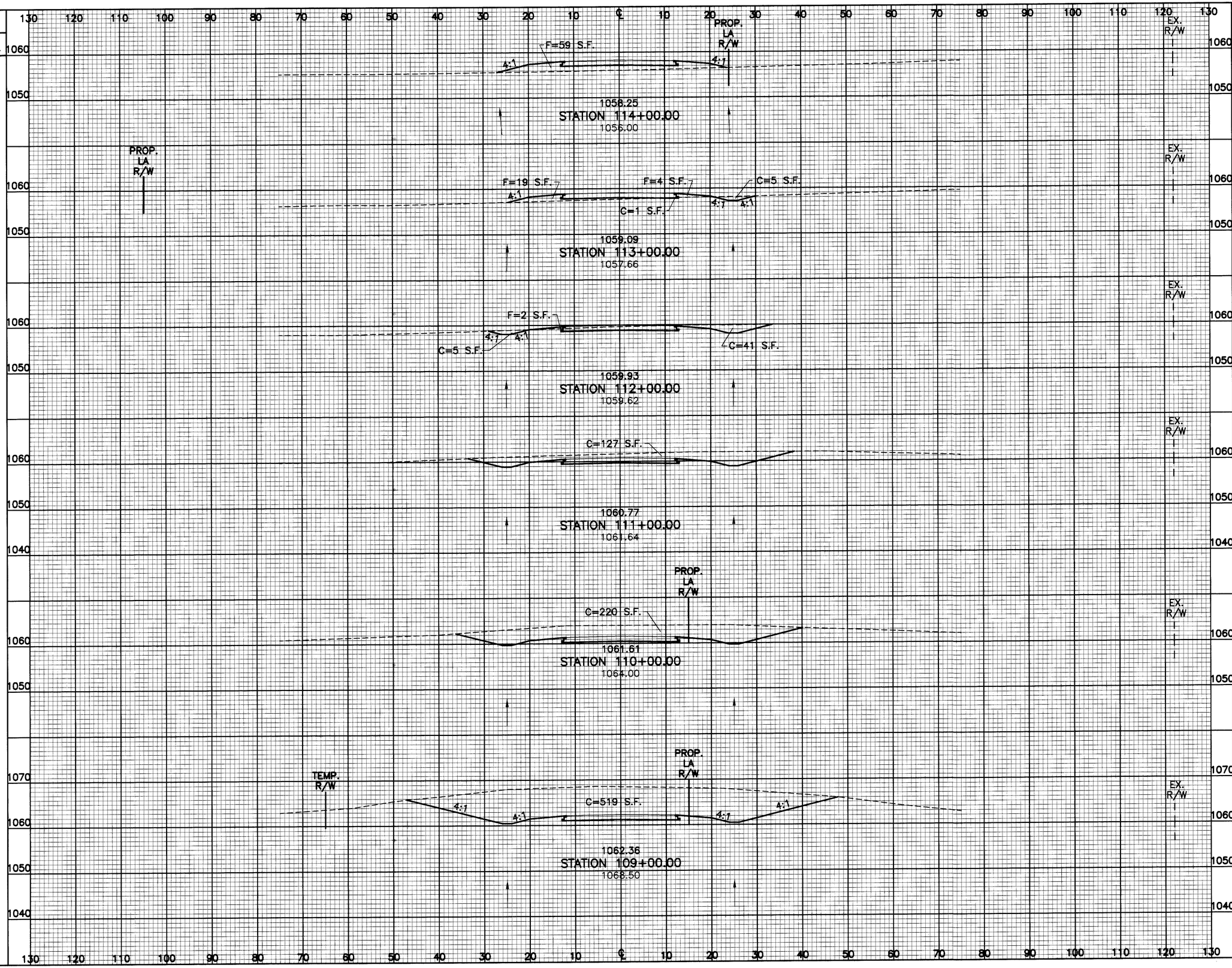
FAY-35-2.57

3590TX2.DWG

SEEDING	
END WIDTH	SQ. YDS.

END AREA		VOLUME	
CUT	FILL	CUT	FILL

CALCULATED J.L.C.
CHECKED W.G.Y.



TEMPORARY ROAD X-SECTIONS STATION 109+00.00 TO STATION 114+00.00

FAY-35-2.57

3590TX3.DWG

SEEDING		END AREA		VOLUME		CALCULATED J.L.C.	CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL		

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120 130

1060 / 1050

PROP. R/W — EX. R/W — PROP. LA R/W — SERVICE RD. 3

SERVICE RD.#3 DITCH — VARIES — 4:1 — 1053.1 41.08'

1059.04
STATION 121+00.00
1055.39

0.2%

PROP. R/W — EX. R/W — PROP. LA R/W — SERVICE RD. 3

4:1 — 4:1 — VARIES — 1053.3 36.72'

1058.65
STATION 120+00.00
1054.29

PROP. R/W — EX. R/W — PROP. LA R/W — SERVICE RD. 3

STA. 119+34, TEMP. RD. =
STA. 19+34, SERV. RD. 3
CONSTRUCT S-21, 30" CULVERT
℄ INLET = 1053.55

4:1 — VARIES — 1053.5 36.13'

1058.21
STATION 119+00.00
1054.20

0.2%

PROP. R/W — EX. R/W — PROP. LA R/W — SERVICE RD. 3

4:1 — 4:1 — 1053.7 46.0'

1057.77
STATION 118+00.00
1055.29

0.2%

PROP. R/W — EX. R/W — PROP. LA R/W — SERVICE RD. 3

C=7 S.F. — C=2 S.F.

4:1 — 4:1 — 1053.9 80'

1054.0

1057.33
STATION 117+00.00
1055.74

EX. SAN. M.H. (LIFT STATION)
STA. 116+36.63, 83.32' LT.
TOP = 1088.2
℄ 4" = 1053.0
℄ 8" = 1044.0

SERVICE RD.#3 DITCH — PROP. R/W — EX. R/W — PROP. LA R/W

C=2 S.F. — F=27 S.F. — C=3 S.F.

4:1 — 4:1 — 1054.1 106'

Ex. 4" SAN.

1054.8

1057.07
STATION 116+00.00
1055.51

PROP. R/W — EX. R/W — PROP. LA R/W

F=50 S.F.

4:1 — 4:1 — 1054.3 109'

Ex. 4" SAN.

1057.43
STATION 115+00.00
1055.32

3590TX4.DWG

ROAD X-SECTIONS STATION 115+00.00 TO STATION 121+00.00

TEMPORARY ROAD

FAY-35-2.57

SEEDING	
END WIDTH	SQ. YDS.

END AREA		VOLUME	
CUT	FILL	CUT	FILL

CALCULATED J.L.C.

CHECKED W.G.Y.

TEMP. R/W

EX. R/W

F=112 S.F.

C=16 S.F.

0.2%

4:1

4:7

73 L.F. - 18" TEMP. PIPE

1051.19
40.40'

1057.01
STATION 130+00.00
1053.86

1053.0
33.6'

C=39 S.F.

TEMPORARY ROAD DITCH

F=57 S.F.

C=21 S.F.

VARIES

SERVICE RD. 3

C=5 S.F.

PROP. LA R/W

1051.47
47'

1057.21
STATION 129+00.00
1054.20

+50

C=2 S.F.

F=59 S.F.

SERVICE ROAD 3 DITCH

1051.7
73.6'

1057.41
STATION 128+00.00
1054.64

PROP. R/W

F=3 S.F.

C=19 S.F.

1051.9
57.21'

1057.61
STATION 127+00.00
1056.77

C=31 S.F.

1052.1
46.11'

1057.81
STATION 126+00.00
1057.57

1058.01
STATION 125+00.00
1056.58

1052.3
40.86'

1058.31
STATION 124+00.00
1056.04

1052.5
40.56'

1058.71
STATION 123+00.00
1054.64

1052.7
41.36'

DRAIN TO CULVERT S-22

SERVICE RD. 3 DITCH

1059.06
STATION 122+00.00
1054.63

1052.9
41.96'

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120 130

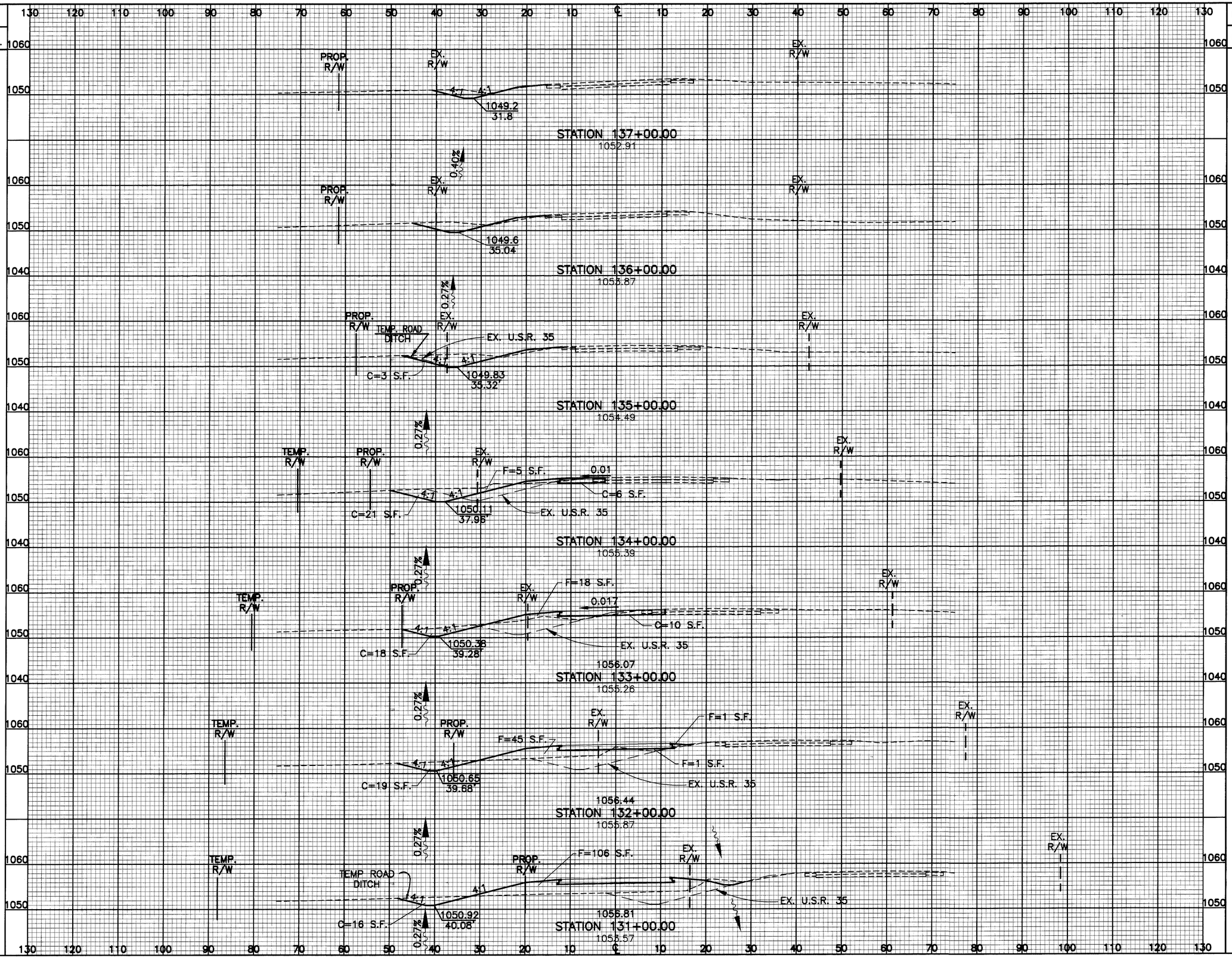
1060 1050

3590TX5.DWG

TEMPORARY ROAD X-SECTIONS STATION 122+00.00 TO STATION 130+00.00

FAY-35-2.57

SEEDING		END AREA		VOLUME		CALCULATED J.L.C.	CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL		



TEMPORARY ROAD X-SECTIONS STATION 131+00.00 TO STATION 137+00.00

FAY-35-2.57

3590TX6.DWG

SHEET NUMBER																			ITEM	ITEM EXT.	GRAND TOTAL	UNIT	DESCRIPTION
12	12A	13	38	39	40	41	42	43	48B	49	50	51	52	53	53A	279	221A	312A					
																							ROADWAY
LUMP																			201	11000	LUMP		CLEARING AND GRUBBING
				LUMP															202	11000	LUMP		STRUCTURE REMOVED
				2224															202	23900	2224	SQ YD	CONCRETE BASE REMOVED
				1799													346		202	35100	2145	LIN FT	PIPE REMOVED, 24" AND UNDER
				2															202	58100	2	EACH	CATCH BASIN REMOVED
				1															202	58700	1	EACH	MANHOLE ABANDONED
																	40		202	75000	40	LIN FT	FENCE REMOVED
				1															SPECIAL	20266000	1	EACH	DRILLED WATER WELL ABANDONED (SEE SHEET NO. 13)
				1															SPECIAL	20298100	1	EACH	REMOVAL MISC : OIL STORAGE TANK
				20			325882		110548										203	12000	436,450	CU YD	EXCAVATION NOT INCLUDING EMBANKMENT CONSTRUCTION
									41672										203	13100	41,672	CU YD	EXCAVATION OF UNSUITABLE MATERIAL
				20			1475672		132220										203	20000	1,607,912	CU YD	EMBANKMENT
							92												203	45000	92	HOUR	PROOF ROLLING
								275553											203	50000	275,553	SQ YD	SUBGRADE COMPACTION
									25000										206	10200	25,000	SQ YD	LIME-SOIL STABILIZED SUBGRADE - DRY OR SLURRY METHOD
									375										206	10300	375	TONS	LIME
									200										206	10400	200	MGAL	WATER
																		7	601	11000	7	SQ YD	RIPRAP USING 6" REINFORCED CONCRETE SLAB
					1252														601	20000	1252	SQ YD	CRUSHED AGGREGATE SLOPE PROTECTION
																148		183	601	32100	331	CU YD	ROCK CHANNEL PROTECTION, TYPE B WITH FILTER
		120			268											157			601	32200	545	CU YD	ROCK CHANNEL PROTECTION, TYPE C WITH FILTER
					104										50				601	32300	154	CU YD	ROCK CHANNEL PROTECTION, TYPE D WITH FILTER
		13200								140	390								601	34200	13,730	CU YD	ROCK CHANNEL PROTECTION, TYPE C WITHOUT FILTER
							17												604	38500	17	EACH	MONUMENT ASSEMBLY
							29												604	40500	29	EACH	REFERENCE MONUMENT
			16412.5																606	13000	16,412.5	LIN FT	GUARDRAIL, TYPE 5
			525																606	15500	525	LIN FT	GUARDRAIL, BARRIER DESIGN, TYPE 5
			12																606	22000	12	EACH	ANCHOR ASSEMBLY, TYPE B-98
			9																606	22010	9	EACH	ANCHOR ASSEMBLY, TYPE E-98
			4																606	25000	4	EACH	ANCHOR ASSEMBLY, TYPE A
			13																606	26500	13	EACH	ANCHOR ASSEMBLY, TYPE T
			24																606	35000	24	EACH	BRIDGE TERMINAL ASSEMBLY, TYPE 1
			4																606	35100	4	EACH	BRIDGE TERMINAL ASSEMBLY, TYPE 2
			6																606	60010	6	EACH	IMPACT ATTENUATOR, TYPE 1-98 (BIDIRECTIONAL)
							55,048												607	15000	55,048	LIN FT	FENCE, TYPE 47
	LUMP																		607	98200	LUMP		FENCE, MISC: CONSTRUCTION FENCE
						1098													660	20000	1098	SQ YD	SODDING REINFORCED
					6061	178													667	10000	6239	SQ YD	SEEDING & JUTE MATTING
					26983	34576											464		670	40000	62,023	SQ YD	DITCH EROSION PROTECTION
				3															SPECIAL	69050100	3	EACH	MAILBOX SUPPORT SYSTEM, SINGLE (SEE SHEET NO. 12A)
					283														673	10000	283	SQ.YD.	SEEDING AND EROSION CONTROL BLANKET, TYPE 'E'
																							EROSION CONTROL
17																			870	00100	17	EACH	SOIL ANALYSIS TEST
45516																			870	00200	45516	CU YD	PLACING TOPSOIL
				119			818520												870	10000	818,639	SQ YD	SEEDING AND MULCHING
		44,299																	870	14000	44,299	SQ YD	REPAIR SEEDING AND MULCHING
41000																			870	15000	41000	SQ YD	INTER-SEEDING
		38					110												870	20000	148	TON	COMMERCIAL FERTILIZER
							746												870	30000	746	TON	AGRICULTURAL LIME
4421		316																	870	35000	4737	M GAL	WATER
1842																			870	40000	1842	M SQ FT	MOWING
		177,197																	877	10000	177,197	SQ YD	TEMPORARY SEEDING AND MULCHING
		11250								4930	5925	2008	3530	150	1569				877	30000	29362	LIN FT	TEMPORARY PERIMETER DITCH CHECK
																							OR INLET PROTECTION FILTER FABRIC FENCE
		2585																	877	40000	2585	LIN FT	TEMPORARY SLOPE DRAIN
		66000								690	1950								877	50000	68,640	CU YD	TEMPORARY SEDIMENT BASINS & DAMS
		10340																	877	55000	10340	CU YD	TEMPORARY DIKES
		16500								173	98								877	60000	16,771	CU YD	SEDIMENT REMOVAL
		15,000																	877	55500	15,000	SQ YD	TEMPORARY DITCH PROTECTION

CALCULATED J.L.G. CHECKED W.G.Y.

REVISED 1/00 BY FYC

GENERAL SUMMARY

FAY-35-2.57

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\2578.75\40lnewsht\new033.dgn 20-MAR-2001 3:34PM dwise
i:\Projects\fay\035\0257.000\033.dgn 15-AUG-2000 1:52PM dwise

i:\pr\project\fay35\fay-35-2.57\ 033.dgn

GENERAL SUMMARY

ITEM	SHEET NUMBERS														ITEM	ITEM EXTENSION	GRAND TOTAL	UNIT	DESCRIPTION
	12	13	14	37	38	39	40	41	42	43	49	50	279	312A					
																			DRAINAGE
602													57.0		602	20000	57.0	CU.YD.	CONCRETE MASONRY
602														5.7	602	20001	5.7	CU.YD.	CONCRETE MASONRY, AS PER PLAN (SEE SHEET 312B)
603		300													603	00100	300	LIN.FT.	4" CONDUIT, TYPE B
603		1000													603	00400	1000	LIN.FT.	4" CONDUIT, TYPE E
603		300													603	00406	300	LIN.FT.	4" CONDUIT, TYPE F
603		300		348									2854		603	00900	3502	LIN.FT.	6" CONDUIT, TYPE B
603		1000													603	01400	1000	LIN.FT.	6" CONDUIT, TYPE E
603		300		39											603	01500	339	LIN.FT.	6" CONDUIT, TYPE F
603													4037		603	01500	4037	LIN.FT.	6" CONDUIT, TYPE F, 707.41
603		300													603	01800	300	LIN.FT.	8" CONDUIT, TYPE B
603		1000													603	02500	1000	LIN.FT.	8" CONDUIT, TYPE E
603		300		39											603	02600	339	LIN.FT.	8" CONDUIT, TYPE F
603		300		200											603	03100	500	LIN.FT.	10" CONDUIT, TYPE B
603		500													603	03600	500	LIN.FT.	10" CONDUIT, TYPE E
603		300		35											603	03700	335	LIN.FT.	10" CONDUIT, TYPE F
603		300		356											603	04400	656	LIN.FT.	12" CONDUIT, TYPE B
603				465											603	04900	465	LIN.FT.	12" CONDUIT, TYPE D
603		500													603	05100	500	LIN.FT.	12" CONDUIT, TYPE E
603		300													603	05200	300	LIN.FT.	12" CONDUIT, TYPE F
603													70		603	05700	70	LIN.FT.	15" CONDUIT, TYPE A, 706.01, 707.01(.109), 707.01 ALUMINUM COATED(.079), OR 707.21
603													64		603	05700	64	LIN.FT.	15" CONDUIT, TYPE A 706.01 OR 18" CONDUIT TYPE A 707.01(.109), 707.01 ALUMINUM COATED,(.079), OR 707.21
603		300		336									2262		603	05900	2898	LIN.FT.	15" CONDUIT, TYPE B
603		500													603	06600	500	LIN.FT.	15" CONDUIT, TYPE E
603		300		105									407		603	06700	812	LIN.FT.	15" CONDUIT, TYPE F
603													89		603	07200	89	LIN.FT.	18" CONDUIT, TYPE A, 706.01 OR 706.02 OR 21" CONDUIT, TYPE A, 707.01 OR 707.21
603													80		603	07200	80	LIN.FT.	18" CONDUIT, TYPE A, 706.01 OR 706.02 OR 24" CONDUIT, TYPE A, 707.01
603													90		603	07400	90	LIN.FT.	18" CONDUIT, TYPE B
603				38											603	07900	38	LIN.FT.	18" CONDUIT, TYPE D
603													85		603	08700	85	LIN.FT.	21" CONDUIT, TYPE A, 706.02 OR 706.08 OR 27" CONDUIT, TYPE A, 707.05
603													875		603	09100	875	LIN.FT.	21" CONDUIT, TYPE C
603													122		603	10200	122	LIN.FT.	24" CONDUIT, TYPE A, 706.02, 707.01(.079),707.01 ALUMINUM COATED(.064), 707.05 OR 707.21(.060)
603													76		603	10200	76	LIN.FT.	24" CONDUIT, TYPE A, 706.02 OR 706.08 OR 27" CONDUIT, TYPE A, 707.05
603													199		603	10200	199	LIN.FT.	24" CONDUIT, TYPE A, 706.02, 707.01(.109),707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603													150		603	10200	150	LIN.FT.	24" CONDUIT, TYPE A, 706.02, 1500 D-LOAD, 707.01(.079), 707.01 ALUMINUM COATED(.064), 707.05, OR 707.21(.060)
603													186		603	10400	186	LIN.FT.	24" CONDUIT, TYPE B
603													63		603	13200	63	LIN.FT.	30" CONDUIT, TYPE A, 706.02
603													196		603	13200	196	LIN.FT.	30" CONDUIT, TYPE A, 706.02, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.075)
603													223		603	13200	223	LIN.FT.	30" CONDUIT, TYPE A, 706.02, 1250 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603													157		603	13200	157	LIN.FT.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.075)
603													180		603	13200	180	LIN.FT.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, OR 36" CONDUIT,TYPE A,707.01 (.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603													114		603	13200	114	LIN.FT.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, OR 42" CONDUIT,TYPE A, 707.01 ALUMINUM COATED(.109), 707.05, OR 707.21(.135)
603													210		603	13200	210	LIN.FT.	30" CONDUIT, TYPE A, 706.02, OR 48" CONDUIT,TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603													183		603	13400	183	LIN.FT.	30" CONDUIT, TYPE B
603													1500		603	13600	1500	LIN.FT.	30" CONDUIT, TYPE C

REVISED 1/00 CHECKED FYC

GENERAL SUMMARY

FAY-35-2.57

i:\users\dwise\fay\35\257&75\40newsht\new034.dgn 20-MAR-2001 3:35PM dwise
i:\pr\project\fay35\fay-35-2.57\034.dgn 01-FEB-2000 2:56PM fcheek
i:\pr\project\fay35\fay-35-2.57\034.dgn
i:\pr\project\fay35\fay-35-2.57\ 034.dgn

REVISED 1-31-01 DSW

ITEM	SHEET NUMBER										ITEM	ITEM EXT.	GRAND TOTAL	UNIT	DESCRIPTION	SEE SHEET NO.
							37	42	312A	279						
															DRAINAGE (CONTINUED)	
512									314		512	33000	314	SQ.YD.	TYPE 2 WATERPROOFING	312B
512									30		512	67502	30	SQ.YD.	SEALING OF CONCRETE SURFACES (EPOXY)	312B
601									7		601	11000	7	SQ.YD.	RIPRAP USING 6" REINFORCED CONCRETE SLAB	
601									183		601	32104	183	CU.YD.	ROCK CHANNEL PROTECTION, TYPE B W/FABRIC FILTER	
602																312B
603										100	603	16200	100	LIN. FT.	36" CONDUIT, TYPE A, 706.02, OR 706.08, OR 48" CONDUIT, TYPE A, 707.05	
603										206	603	16200	206	LIN. FT.	36" CONDUIT, TYPE A, 706.02 OR 54" CONDUIT, TYPE A, 707.02 (.109), 707.02 FIELD PAVED (.064), 707.02 ALUMINUM COATED (.064), 707.07(.079), OR 707.22	
603										223	603	16200	223	LIN. FT.	36" CONDUIT, TYPE A, 706.02 1500 D-LOAD, OR 48" CONDUIT TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)	
603										224	603	16200	224	LIN. FT.	36" CONDUIT, TYPE A, 706.02 2000 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED (.064), 707.05(.079), OR 707.21(.105)	
603										268	603	16200	268	LIN. FT.	36" CONDUIT, TYPE A, 706.02 2500 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.079), 707.05(.079), OR 707.21(.105)	
603										720	603	16600	720	LIN. FT.	36" CONDUIT, TYPE C	
603										263	603	19200	263	LIN. FT.	42" CONDUIT, TYPE A, 706.02 2500 D-LOAD OR 66" CONDUIT, TYPE A, 707.02 FIELD PAVED (.064), 707.02(.109), 707.02 ALUMINUM COATED(.064), 707.04, 707.07(.079), OR 707.22	
603										200	603	20700	200	LIN. FT.	48" CONDUIT, TYPE A, 706.02 OR 66" CONDUIT, TYPE A, 707.02 FIELD PAVED(.064), 707.02(.109), 707.02 ALUMINUM COATED(.079), 707.04, 707.07(.079), 707.22	
603										196	603	22200	196	LIN. FT.	54" CONDUIT, TYPE A, 706.02	
603										211	603	23600	211	LIN. FT.	60" CONDUIT, TYPE A, 706.02	
603										224	603	52400	224	LIN. FT.	22" x 34" CONDUIT, TYPE A, 706.04, HE-II	
603										219	603	53200	219	LIN. FT.	48" x 76" CONDUIT, TYPE A, 706.04, HE-I	
603									140		603	94801	140	LIN. FT.	8' x 4' CONDUIT, TYPE A, 706.05, AS PER PLAN	312B
603										290	603	98300	290	LIN. FT.	MISC. CONDUIT: 4" CONDUIT, TYPE B, FOR SANITARY (SEE SHEET 13)	
604										4	604	00800	4	EACH	CATCH BASIN, NO. 3A	
604										23	604	01200	23	EACH	CATCH BASIN, NO. 4	
604										1	604	01400	1	EACH	CATCH BASIN, NO. 4A	
604										3	604	02800	3	EACH	CATCH BASIN, NO. 8	
604										9	604	02906	9	EACH	CATCH BASIN, NO. 8A	
604										2	604	04500	2	EACH	CATCH BASIN, NO. 2-2B	
604							1				604	31500	1	EACH	MANHOLE, NO. 3	
604										68	604	36600	68	EACH	PRECAST REINFORCED CONCRETE OUTLET	
605										4516	605	05100	4516	LIN. FT.	4" SHALLOW PIPE UNDERDRAIN	
605										100,731	605	05111	100,731	LIN. FT.	4" SHALLOW PIPE UNDERDRAIN, WITH FABRIC WRAP, AS PER PLAN	6
605										98565	605	11100	98565	LIN. FT.	6" SHALLOW PIPE UNDERDRAIN	
605										9952	605	12200	9952	LIN. FT.	6" DEEP PIPE UNDERDRAIN	
605										2782	605	13300	2782	LIN. FT.	6" UNCLASSIFIED PIPE UNDERDRAIN	
605								3214			605	31100	3214	LIN. FT.	AGGREGATE DRAIN	

REVISED 11/99 BY FYC

GENERAL SUMMARY

FAY-35-2.57

35
522

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&75\40lnewsht\new035.dgn 20-MAR-2001 3:35PM dwise
i:\pr\project\fay35\fay-35-2.57\035.dgn 01-FEB-2000 2:47PM fcheek

SHEET NUMBER											ITEM	ITEM EXT.	GRAND TOTAL	UNIT	DESCRIPTION	SEE SHEET NO.
ITEM	12b	13	14	37	38	39	43	279								
															PAVEMENT	
301			15				5766	9			301	46000	5,790	CU.YD.	BITUMINOUS AGGREGATE BASE, PG64-22	
304						423	50687				304	20000	50,817	CU.YD.	AGGREGATE BASE	
307							229781				307	10100	229,781	SQ.YD.	4" NON-STABILIZED DRAINAGE BASE, TYPE IA	
408						475	100549				408	10000	100,523	GAL.	BITUMINOUS PRIME COAT	
407							364				407	14000	364	GAL.	TACK COAT FOR INTERMEDIATE COURSE	
411							89				411	10000	89	CU.YD.	STABILIZED CRUSHED AGGREGATE	
446							1162				446	46050	1,162	CU.YD.	ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22	
446							830				446	47020	830	CU.YD.	ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22	
448							61	3			448	46050	3	CU.YD.	ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22	
448			1020				394	2			448	47020	1,373	CU.YD.	ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22	
448						27					448	48020	27	CU.YD.	ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22 (DRIVEWAYS)	
609				948							609	26000	948	LIN.FT.	CURB, TYPE 6	
611							1306				611	15001	1,306	SQ.YD.	REINFORCED CONCRETE APPROACH SLAB (T-13"), AS PER PLAN	13
611							962				611	25001	962	SQ.YD.	REINFORCED CONCRETE APPROACH SLAB (T-15"), AS PER PLAN	13
880							72333				880	15000	72,333	CU.YD.	ASPHALT CONCRETE (7 YEAR WARRANTY)	13
															FOR TRAFFIC CONTROL GENERAL SUMMARY, SEE SHEET 354	
															FOR LIGHTING GENERAL SUMMARY, SEE SHEET 386	
															LANDSCAPING	
	225										661	99900	225	EACH	PLANTING MISC.: DECIDUOUS TREE, 2-3' HEIGHT	12b
															BUILDING DEMOLITION	
											202	56000	LUMP		PARCEL 161-WL, REMOVAL 1-STORY FRAME RESIDENCE	
											202	56000	LUMP		PARCEL 172-WL, REMOVAL 5 METAL SHEDS	
											202	56000	LUMP		PARCEL 178-WL, REMOVAL 1 POLE BARN	
															MAINTENANCE OF TRAFFIC	
407			2,420								407	10000	2,420	GAL.	TACK COAT	
410			500								410	12000	500	CU.YD.	TRAFFIC COMPACTED SURFACE, TYPE A OR B	
410			100								410	13000	100	CU.YD.	TRAFFIC COMPACTED SURFACE, TYPE C	
614			60								614	11100	60	HOUR	LAW ENFORCEMENT OFFICER WITH PATROL CAR	
SPECIAL			40								SPECIAL	614 12500	40	SQ.FT.	REPLACEMENT SIGN	15
SPECIAL			25								SPECIAL	614 12600	25	EACH	REPLACEMENT DRUM	15
614			80								614	13000	80	CU.YD.	BITUMINOUS CONCRETE FOR MAINTAINING TRAFFIC	
614			145								614	13300	145	EACH	BARRIER REFLECTOR, TYPE B	15
614			145								614	13350	145	EACH	OBJECT MARKER	15
614			0.66								614	21100	0.66	MILE	TEMPORARY CENTER LINE, CLASS I, 642 PAINT	
614			0.13								614	21200	0.13	MILE	TEMPORARY CENTER LINE, CLASS I, 740.06, TYPE I	
614			1.31								614	22100	1.31	MILE	TEMPORARY EDGE LINE, CLASS I, 642 PAINT	
614			2.79								614	22200	2.79	MILE	TEMPORARY EDGE LINE, CLASSI, 740.06, TYPE I	
614			5,820								614	23400	5,820	LIN.FT.	TEMPORARY CHANNELIZING LINE, CLASS I, 740.06, TYPE I	
615			LUMP								615	10000	LUMP		TEMPORARY ROAD	
615			9,934								615	25000	9,934	SQ.YD.	TEMPORARY PAVEMENT, CLASS B	
616			210								616	10000	210	M. GAL.	WATER	
616			30								616	20000	30	TON	CALCIUM CHLORIDE	
617			170								617	10100	170	CU.YD.	COMPACTED AGGREGATE, TYPE A	
617			17								617	25000	17	M. GAL.	WATER	
622			3,720								622	40020	3,720	LIN.FT.	PORTABLE CONCRETE BARRIER, 32"	
642			4								642	00290	4	MILE	CENTER LINE	
															STRUCTURES OVER 20 FEET	
															FOR FAY-35-0258, SEE SHEETS 393-403	
															FOR FAY-35-0308, SEE SHEETS 404-416	
															FOR FAY-35-0366, SEE SHEETS 417-436	
															FOR FAY-35-0435, SEE SHEETS 437-454	
															FOR FAY-35-0518, SEE SHEETS 455-466	
															FOR FAY-35-0644, SEE SHEETS 467-482	
614			LUMP								614	11000	LUMP		MAINTAINING TRAFFIC	
623											623	10000	LUMP		CONSTRUCTION LAYOUT STAKES	
624											624	10000	LUMP		MOBILIZATION	
806											806	16020	24	MONTH	FIELD OFFICE, TYPE C	
806																

REVISED 12/99 BY FYC

GENERAL SUMMARY

FAY-35-2.57

i:\users\dwise\fay\35\2578.75\40lnewsht\new036.dgn 20-MAR-2001 3:35PM dwise
i:\pr\project\fay35\fay-35-2.57\fay36gg.dgn 01-FEB-2000 2:53PM faheek
i:\pr\project\fay35\fay-35-2.57\ fay36gg.dgn

REVISED 1-31-01 DSW

SUB – SUMMARY

* See General Note on Sht. No. 13, "Sanitary Work"

REF. NO.	SHT. NO.	STATION TO STATION	SIDE	603 * 4" CONDUIT TYPE B FOR SANITARY L.F.	603 6" CONDUIT TYPE B L.F.		603 6" CONDUIT TYPE F L.F.		603 10" CONDUIT TYPE B L.F.	603 8" CONDUIT TYPE F L.F.	603 6" CONDUIT TYPE B L.F.	603 10" CONDUIT TYPE F L.F.		603 12" CONDUIT TYPE D L.F.				603 15" CONDUIT TYPE B L.F.	603 15" CONDUIT TYPE F L.F.		603 18" CONDUIT TYPE D L.F.		604 MANHOLE, NO.3 EACH		609 CURB TYPE 6 L.F.		
1-D	55	142+87 TO 142+91	LT																10								
3-D	64	279+45 TO 281+14	LT/RT															336					1				
5-D	69	346+83	RT									35															
7-D	70	360+26	RT																62								
8-D	72	388+90	RT				39																				
9-D	183	63+61 TO 63+95, EX35	LT/RT						200																		
10-D	184	86+63 TO 87+18, EX35	LT/RT		348																						
11-D	184	97+81 TO 98+19, EX35	LT																		38						
12-D	222	11+37.7 TO 11+90.6, SERV RD NO 1	LT											46													
23-D	59	205+90 TO 206+45	LT/RT								356																
13-D	237	155+00 TO 156+01.2, SR 729	LT											102													
14-D	237	155+63.1 TO 156+59.8, SR 729	RT											97													
15-D	237	161+74.1 TO 162+67, SR 729	RT											93													
16-D	239	177+77.6 TO 178+20.4, SR 729	LT											43													
21-D	229	15+35 TO 15+85, Serv. Rd. No. 3	LT											50													
22-D	229	13+45 TO 16+35, Serv. Rd. No. 3	LT	290																							
17-D	163	38+54 TO 38+88, C.R. 16	LT											34													
18-D	66	301+75	RT							39																	
19-D	66	308+00	RT																33								
20-D	61	227+46.13	RT																				1				
1-C	54	136+60.31 TO 136+80.31	LT																						20		
2-C	54	137+34.25 TO 137+58.25	RT																						24		
3-C	163	31+63.75 TO 31+86.75, C.R. 16	LT																						23		
4-C	163	31+45.68 TO 31+68.68, C.R. 16	RT																						23		
5-C	162	28+31.32 TO 28+54.32, C.R. 16	LT																						23		
6-C	162	28+13.25 TO 28+36.25, C.R. 16	RT																						23		
7-C	58	193+06.80 TO 193+30.80	LT																						24		
8-C	58	196+32.96 TO 196+56.96	LT																						24		
9-C	58	193+04.76 TO 193+28.76	RT																						24		
10-C	58,59	196+37.00 TO 200+86.00	RT																						449		
11-C	184	78+54.69 TO 78+79.69, EX35	LT																						25		
12-C	184	79+06.65 TO 79+37.65, EX35	RT																						31		
13-C	184	75+22.13 TO 75+53.13, EX35	LT																						31		
14-C	184	75+81.09 TO 76+05.09, EX35	RT																						24		
15-C	238	168+50.37 TO 168+74.37, SR 729	LT																						24		
16-C	238	168+40.00 TO 168+64.00, SR 729	RT																						24		
17-C	238	165+34.27 TO 165+58.27, SR 729	LT																						24		
18-C	238	165+23.91 TO 165+47.91, SR 729	RT																						24		
19-C	69	340+89.12 TO 341+07.12	LT																						18		
20-C	69	343+11.70 TO 343+35.70	LT																						24		
21-C	69	339+59.18 TO 339+83.18	RT																						24		
22-C	69	341+87.76 TO 342+05.76	RT																						18		
TOTALS				290	348		39		200	39	356	35		465				336	105		38		1		948		

CALCULATED | CHECKED

ESTIMATED QUANTITIES

FAY–35–2.57

SUB – SUMMARY

REF. NO.	SHT. NO.	STATION TO STATION	SIDE		606 GUARDRAIL TYPE 5 L.F.	606 GUARDRAIL BARRIER DESIGN TYPE 5 L.F.	606 IMPACT ATTENUATOR TYPE 1–98 (BIDIRECTIONAL) EACH	606 ANCHOR ASSEMBLY TYPE A, EACH	606 ANCHOR ASSEMBLY TYPE B–98 EACH	606 ANCHOR ASSEMBLY TYPE E–98 EACH	606 ANCHOR ASSEMBLY TYPE T EACH		606 BRIDGE TERMINAL ASSEMBLY TYPE 1 EACH	606 BRIDGE TERMINAL ASSEMBLY TYPE 2 EACH
1–GR	54	871+11.87BK TO 137+30.91AHD	RT		181.25				1				1	
2–GR	54	871+28.84BK TO 137+60.35AHD	RT		193.75				1				1	
3–GR	54	138+39.65 TO 140+44.98	LT		193.75				1				1	
4–GR	54	138+69.09 TO 140+61.95	LT		181.25				1				1	
5–GR	56	160+94.43 TO 162+88	LT/RT		62.5	87.5	1				1			
6–GR	56	162+30.00 TO 164+23.57	LT/RT		62.5	87.5	1				1			
7–GR	57,58	182+60.91 TO 193+32.53	LT		1075						1			1
8–GR	58	191+38.84 TO 193+32.18	RT		193.75				1				1	
9–GR	57,58	182+85.18 TO 193+31.24	RT		1018.75				1				1	
10–GR	58,59	196+32.01 TO 211+64.61	LT		1543.75				1				1	
11–GR	58,59	196+34.57 TO 198+26.98	LT		193.75				1				1	
12–GR	58,59	196+36.08 TO 210+31.85	RT		1362.5						1			1
13–GR	61	229+82.68 TO 232+75.79	LT/RT		162.5	87.5	1				1			
14–GR	61	231+14.40 TO 234+20.80	LT/RT		175	87.5	1				1			
15–GR	64	271+69.49 TO 273+63.06	LT/RT		62.5	87.5	1				1			
16–GR	64	273+10.56 TO 275+04.13	LT/RT		62.5	87.5	1				1			
17–GR	68,69	333+46.16 TO 341+08.66	LT		750						1			1
18–GR	69	338+39.07 TO 340+31.93	RT		193.75				1				1	
19–GR	68,69	333+29.95 TO 339+85.28	RT		643.75				1				1	
20–GR	69	343+09.60 TO 350+58.24	LT		731.25				1				1	
21–GR	69	342+62.95 TO 344+55.81	LT		193.75				1				1	
22–GR	69,70	341+86.22 TO 352+40.92	RT		1050						1			1
23–GR	162	26+37.40 TO 28+56.15, C.R. 16	LT		193.75			1					1	
24–GR	162	26+19.60 TO 28+38.35, C.R. 16	RT		193.75			1					1	
25–GR	163	31+61.65 TO 32+62.33, C.R. 16	LT		93.75			1					1	
26–GR	163	31+43.85 TO 32+62.60, C.R. 16	RT		93.75			1					1	
27–GR	183,184	68+86.48 TO 75+55.23, EX35	LT		643.75					1			1	
28–GR	184	75+00.50 TO 76+06.75, EX35	RT		81.25					1			1	
29–GR	183,207 208	68+02.98, EX35 TO 235+11.05, RAMP C	LT		1112.5					1	1			
30–GR	184	78+53.03 TO 84+96.01, EX35	LT		618.75					1			1	
31–GR	184	79+04.55 TO 82+99.00, EX35	RT		368.75					1			1	
32–GR	237	156+92.11 TO 162+23.69, SR 729	LT		512.5					1	1			
33–GR	237,238	162+49.25 TO 165+60.23, SR 729	LT		306.25						1		1	
34–GR	237,238	156+93.91 TO 165+50.16, SR 729	RT		831.25					1			1	
35–GR	238	168+48.11 TO 174+04.39, SR 729	LT		531.25					1			1	
36–GR	238	168+38.05 TO 174+06.77, SR 729	RT		543.75					1			1	
TOTALS					16412.5	525	6	4	12	9	13		24	4

CALCULATED | CHECKED

ESTIMATED QUANTITIES

FAY–35–2.57

SUB – SUMMARY

REF. NO.	SHT. NO.	STATION TO STATION	SIDE		202 STRUCTURE REMOVED LUMP	202 CONCRETE BASE REMOVED S.Y.	202 PIPE REMOVED 24" & UNDER L.F.	202 CATCH BASIN REMOVED EACH	202 MANHOLE ABANDONED EACH	SPECIAL DRILLED WATER WELL ABANDONED EACH	SPECIAL REMOVAL MISC., OIL STORAGE TANK EACH		203 EXCAVATION NOT INCL EMBANK. CONSTR. C.Y.	203 EMBANKMENT C.Y.	659 SEEDING & MULCHING S.Y.			304 AGGREGATE BASE C.Y.				408 BITUMINOUS PRIME COAT GAL.	448 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22 (DRIVEWAYS) C.Y.		SPECIAL MAILBOX SUPPORT SYSTEM, SINGLE EACH	
1-R	66	304+70 TO 305+25	LT				112																			
2-R	66	307+25 TO 308+00	RT				97																			
3-R	69	346+83 TO 347+50	RT				100																			
4-R	70	354+35 TO 355+00	RT				99																			
5-R	70	360+26 TO 361+55	RT				157																			
6-R	71	367+90 TO 368+50	RT				110																			
7-R	72	387+30	RT		LUMP																					
8-R	162	27+49, C.R. 16	LT/RT				40																			
9-R	163	38+61 TO 38+78, C.R. 16	LT				17																			
10-R	174	3+85 TO 4+00, T.R. 94	LT				15																			
11-R	183	63+28 TO 63+58, EX35	LT				30																			
12-R	183	63+64 TO 63+66, EX35	LT/RT				88																			
13-R	183	62+25 TO 66+50, EX35	LT/RT			945																				
14-R	183	67+33 TO 67+74, EX35	RT				41																			
15-R	183	67+95 TO 68+33, EX35	RT				38																			
16-R	183	68+09 TO 68+34, EX35	LT				25																			
17-R	184	76+00 TO 78+75, EX35	LT/RT			612																				
18-R	184	82+73, EX35	LT/RT				50																			
19-R	185	86+95 TO 87+07, EX35	LT/RT				60	2																		
20-R	185	87+82, EX35	LT/RT				46																			
21-R	185	89+05 TO 89+42, EX35	RT				37																			
22-R	185	89+57 TO 89+88, EX35	RT				31																			
23-R	185	87+00 TO 90+00, EX35	LT/RT			667																				
24-R	185	90+11, EX35	LT								1															
25-R	185	90+30, EX35	LT							1																
26-R	185	89+73 TO 93+11, EX35	LT				238																			
27-R	185	98+00 TO 98+20, EX35	LT				20																			
28-R	237	152+99 TO 153+28, SR 729	LT				29																			
29-R	237	155+15 TO 155+48, SR 729	LT				33																			
30-R	237	155+65 TO 155+93, SR 729	LT				28																			
31-R	237	155+18 TO 156+30, SR 729	RT				112																			
32-R	237.	161+61 TO 161+81, SR 729	LT				20																			
33-R	238	171+11 TO 171+51, SR 729	RT				40																			
34-R	183	68+08.15, EX35	RT										20	20	119											
35-R	237	156+32 TO 157+18, SR 729	RT				86																			
36-R	61	227+52	RT						1																	
1-DW	162	23+80, C.R. 16	LT															28				57	8			
2-DW	162	24+40, C.R. 16	RT															25								
3-DW	163	38+70, C.R. 16	LT															10								
4-DW	174	3+91, T.R. 94	LT															4								
5-DW	185	92+25, EX35	RT															9								
6-DW	185	98+00, EX35	LT															12								
7-DW	222	11+50, SERV RD. NO. 1	LT															33								
8-DW	222	12+50.57, SERV. RD. NO. 1	LT															92				46	7			
9-DW	237	153+19, SR 729	LT															11								
10-DW	237	155+31, SR 729	LT															37				33	5			
11-DW	237	155+69, SR 729	LT															29								
12-DW	237	156+04, SR 729	RT															29				32	5			
13-DW	237	162+50, SR 729	LT															53								
14-DW	239	178+00, SR 729	LT															12								
15-DW	229	16+00, SERVICE RD. NO. 3	LT															39				307	2			
1-MB	162	23+85, C.R. 16	LT																						1	
2-MB	222	12+46.7, SERV. RD. NO. 1	LT																						1	
3-MB	237	155+13, SR 729	LT																						1	
TOTALS					LUMP	2224	1799	2	1	1	1		20	20	119			423				475	27		3	

CALCULATED

CHECKED

ESTIMATED QUANTITIES

FAY-35-2.57

SUB – SUMMARY

REF. NO.	SHT. NO.	STATION TO STATION	SIDE	601 CRUSHED AGGREGATE SLOPE PROTECTION C.Y.	601 ROCK CHANNEL PROTECTION TYPE C W/FILTER C.Y.	601 ROCK CHANNEL PROTECTION TYPE D W/FILTER C.Y.	667 SEEDING & JUTE MATTING S.Y.	670 DITCH EROSION PROTECTION S.Y.	673 SEEDING AND EROSION CONTROL BLANKET TYPE 'E' S.Y.
1-EC	54	136+80.31 TO 137+01.62	LT				32		
2-EC	54	136+94.64 TO 137+92.19	LT/RT		126				
3-EC	54	137+58.25 TO 137+94.08	RT				64		
4-EC	54	138+04.04 TO 138+41.75	LT				73		
5-EC	54	138+07.56 TO 139+05.36	LT/RT		142				
6-EC	54	138+96.41 TO 139+19.69	RT				38		
7-EC	54,55	141+32 TO 142+82	CL					125	
8-EC	54,55	139+56 TO 142+96	RT						283
9-EC	55	142+90 TO 144+60	LT			104			
10-EC	55	144+60 TO 152+10	LT					1325	
11-EC	55	150+32 TO 151+82	CL					125	
12-EC	55	144+32 TO 145+82	RT					125	
13-EC	55	147+32 TO 148+82	RT					125	
14-EC	55	150+32 TO 151+82	RT					125	
15-EC	55	153+32 TO 154+82	RT					125	
16-EC	56	156+32 TO 157+82	CL					125	
17-EC	56	156+32 TO 157+82	RT					125	
18-EC	56	159+32 TO 160+82	RT					125	
19-EC	56	31+14.10 TO 31+63.75, C.R. 16	LT				126		
20-EC	56	30+82.14 TO 31+44.18, C.R. 16	LT/RT	83					
21-EC	56	30+78.09 TO 31+44.87, C.R. 16	RT				165		
22-EC	56	28+55.13 TO 29+21.43, C.R. 16	LT				170		
23-EC	56	28+55.88 TO 29+17.53, C.R. 16	LT/RT	82					
24-EC	56	28+36.25 TO 28+86.65, C.R. 16	RT				215		
25-EC	56	162+32 TO 163+82	RT					125	
26-EC	56	165+32 TO 166+82	CL					125	
27-EC	56	165+32 TO 166+82	RT					125	
28-EC	56	168+07 TO 169+57	RT					125	
29-EC	57	171+07 TO 173+93	RT					234	
30-EC	57	174+07 TO 175+57	RT					125	
31-EC	57	174+07 TO 175+57	CL					125	
32-EC	57	181+90 TO 182+48	LT					91	
33-EC	57,58	182+52 TO 184+10	LT					246	
34-EC	57,58	182+57 TO 186+10	CL					393	
35-EC	57,58	183+90 TO 186+10	RT					294	
36-EC	58	193+30.99 TO 193+90.19	LT				467		
37-EC	58	193+51.91 TO 193+90.19	LT/RT	221					
38-EC	58	193+28.05 TO 193+87.46	RT				382		
39-EC	58	195+72.55 TO 196+34.08	LT				425		
40-EC	58	195+72.55 TO 196+16.02	LT/RT	234					
41-EC	58	195+77.71 TO 196+41.09	RT				406		
42-EC	59	200+92 TO 203+60	RT					298	
43-EC	59	203+65 TO 208+93	CL					662	
44-EC	60	217+43 TO 218+93	CL					125	
45-EC	61	227+03 TO 228+53	CL					125	
46-EC	61	227+40 TO 228+60	RT					120	
47-EC	61	75+29.12 TO 75+85.56, EX35	LT				255		
48-EC	61	75+76.50 TO 76+49.30, EX35	LT/RT	45					
49-EC	61	76+06.40 TO 76+66.77, EX35	RT				285		
50-EC	61	77+83.02 TO 78+53.38, EX35	LT				406		
51-EC	61	78+09.40 TO 78+83.30, EX35	LT/RT	38					
52-EC	61	78+77.75 TO 79+30.65, EX35	RT				216		
SUB-TOTALS				703	268	104	3725	5788	283

REF. NO.	SHT. NO.	STATION TO STATION	SIDE	601 CRUSHED AGGREGATE SLOPE PROTECTION C.Y.	601 ROCK CHANNEL PROTECTION TYPE C W/FILTER C.Y.	601 ROCK CHANNEL PROTECTION TYPE D W/FILTER C.Y.	667 SEEDING & JUTE MATTING S.Y.	670 DITCH EROSION PROTECTION S.Y.	673 SEEDING AND EROSION CONTROL BLANKET TYPE 'E' S.Y.
53-EC	61	233+40 TO 234+60	LT					100	
54-EC	61	232+40 TO 233+60	RT					107	
55-EC	61	234+43 TO 235+93	CL					125	
56-EC	61	236+07 TO 237+57	CL					125	
57-EC	62,63	245+90 RAMP D TO 264+60	LT					3314	
58-EC	62,63	252+93 TO 254+43	CL					125	
59-EC	62,63	253+40 TO 254+48	RT					120	
60-EC	63	254+52 TO 260+10	RT					758	
61-EC	63	260+93 TO 262+43	CL					125	
62-EC	63	262+52 TO 263+60	RT					120	
63-EC	64	269+08 TO 270+58	CL					125	
64-EC	64	167+95.34 TO 168+50.82, SR 729	LT					276	
65-EC	64	167+79.09 TO 168+29.60, SR 729	LT/RT	75					
66-EC	64	167+73.60 TO 168+39.54, SR 729	RT					399	
67-EC	64	273+50 TO 274+10	LT					50	
68-EC	64	165+58.73 TO 166+24.25, SR 729	LT					360	
69-EC	64	165+68.65 TO 166+19.18, SR 729	LT/RT						
70-EC	64	165+47.75 TO 166+02.94, SR 729	RT					263	
71-EC	64	277+15 TO 278+60	RT					129	
72-EC	64	275+40 TO 279+98	LT					764	
73-EC	64	280+02 TO 280+85	LT					139	
74-EC	64	278+43 TO 279+93	CL					125	
75-EC	64,65	281+02.5 TO 291+54.5	LT					3040	
76-EC	64,65	280+90 TO 282+10	RT					107	
77-EC	65	290+00 TO 291+50	CL					125	
78-EC	65	291+64 TO 293+14	CL					125	
79-EC	65	290+00 TO 290+50	RT					125	
80-EC	65	291+64 TO 293+14	RT					125	
81-EC	65,66	291+59.5 TO 306+98	LT					4465	
82-EC	66	305+43 TO 306+93	CL					125	
83-EC	66,67	300+90 TO 310+10	RT					1791	
84-EC	66	307+02 TO 309+84.5	LT					916	
85-EC	67	310+15.5 TO 312+98	LT					471	
86-EC	67	313+02 TO 316+47.5	LT					576	
87-EC	67	316+52.5 TO 317+10	LT					96	
88-EC	67	311+43 TO 312+93	CL					125	
89-EC	67	313+07 TO 314+57	CL					125	
90-EC	67	321+07 TO 322+57	CL					125	
91-EC	68	330+07 TO 337+10	CL					391	
92-EC	68,69	335+90 TO 342+26	LT					1626	
93-EC	68,69	337+90 TO 339+83	RT					365	
94-EC	69	339+83.18 TO 340+27.72	RT					175	
95-EC	69	340+06.32 TO 341+01.41	RT	116					
96-EC	69	340+83.15 TO 341+72.48	LT	115					
97-EC	69	341+08.49 TO 341+76.00	LT					351	
98-EC	69	341+19.91 TO 341+86.39	RT					334	
99-EC	69	341+20.99 TO 342+11.05	RT	123					
100-EC	69	341+91.36 TO 342+88.06	LT	120				178	
		SUB-TOTAL		549			2336	21195	
		SUB-TOTAL LEFT SIDE		703	268	104	3725	5788	283
		GRAND TOTALS		1252	268	104	6061	26983	283

CALCULATED

CHECKED

ESTIMATED QUANTITIES

FAY-35-2.57

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&751\40lnewsht\new040.dgn 20-MAR-2001 3:36PM dwise

SUB – SUMMARY

REF. NO.	SHT. NO.	STATION TO STATION	SIDE	660 SODDING REINF. S.Y.	667 SEEDING & JUTE MATTING S.Y.	670 DITCH EROSION PROTECTION S.Y.
101-EC	69	342+67.03 TO 343+11.70	LT		178	
102-EC	69	343+35 TO 351+98	LT			2818
103-EC	69	348+40 TO 351+93	CL			432
104-EC	69,70,71	340+75 TO 368+10	RT			5737
105-EC	70	352+02 TO 361+98	LT			3542
106-EC	70	355+00 TO 361+93	CL			578
107-EC	70,71	362+02 TO 367+95.5	LT			2111
108-EC	71	368+04.5 TO 371+98	LT			1268
109-EC	71,72	372+02 TO 382+23	LT			3290
110-EC	71	370+43 TO 371+93	CL			125
111-EC	72	380+68 TO 382+18	CL			125
112-EC	72	382+27 TO 389+69	LT			2391
113-EC	72	388+40 TO 390+60	RT			221
114-EC	72	389+61 TO 391+11	CL			125
115-EC	72	391+25 TO 392+75	CL			125
116-EC	72	389+81 TO 391+16	LT			525
117-EC	72,73, 329	391+20 TO 14+00, BL CHANNEL NO. 2	LT			3811
118-EC	163	33+90 TO 40+10, C.R. 16	LT			655
119-EC	174	1+90 TO 3+10, T.R. 94	RT			100
120-EC	183,184	70+90 TO 73+00, EX35	LT			175
121-EC	184	74+90 TO 75+25, EX35	LT			30
122-EC	184,185	85+40 TO 86+60, EX35	RT			100
123-EC	230	22+33.50 TO 28+73, SERV. RD. NO. 3	LT			1279
124-EC	185	28+77, SERV. RD. NO. 3 TO 97+81, EX35	LT			1020
125-EC	185,186	98+19 TO 104+00, EX35	LT			1162
126-EC	207,208	233+27.5 TO 235+10, RAMP C	RT			163
127-EC	237	154+40 TO 155+00, SR 729	LT			50
128-EC	237	156+01.5 TO 156+60, SR 729	LT			49
129-EC	237	156+60 TO 157+60, SR 729	RT			84
130-EC	237,238	161+65 TO 164+10, SR 729	RT			205
131-EC	238	167+71.6 TO 170+00, SR 729	RT			254
132-EC	326	0+00 TO 5+90.5, BL CHANNEL NO. 1	LT/RT			2026
133-EC	54	136+52.31 TO 136+61.31	LT	40		
134-EC	54	137+26.25 TO 137+35.25	RT	38		
135-EC	56	28+23.32 TO 28+32.32 C.R.16	LT	34		
136-EC	56	28+05.25 TO 28+14.25 C.R.16	RT	36		
137-EC	56	31+85.75 TO 31+94.75 C.R.16	LT	38		
138-EC	56	31+67.68 TO 31+76.68 C.R.16	RT	45		
139-EC	58	192+98.80 TO 193+07.80	LT	86		
140-EC	58	196+55.96 TO 196+64.96	LT	82		
141-EC	61	75+14.13 TO 75+23.13 EX.35	LT	71		
142-EC	61	75+73.09 TO 75+82.09 EX.35	RT	51		
143-EC	64	165+26.27 TO 165+35.27 S.R.729	LT	78		
144-EC	64	165+15.91 TO 165+24.91 S.R.729	RT	73		
145-EC	64	168+73.37 TO 168+82.37 S.R.729	LT	74		
146-EC	64	168+63.00 TO 168+72.00 S.R.729	RT	74		
TOTALS				820	178	34576

REF. NO.	SHT. NO.	STATION TO STATION	SIDE	660 SODDING REINF. S.Y.	667 SEEDING & JUTE MATTING S.Y.	670 DITCH EROSION PROTECTION S.Y.
147-EC	69	340+81.12 TO 340+90.12	LT	69		
148-EC	69	339+51.18 TO 339+60.18	RT	67		
149-EC	69	343+34.70 TO 343+43.70	LT	72		
150-EC	69	342+04.76 TO 342+13.76	RT	70		
		SUB-TOTAL		278		
		SUB-TOTAL LEFT SIDE		820	178	34576
		GRAND TOTALS		1098	178	34576

CALCULATED

CHECKED

ESTIMATED QUANTITIES

FAY-35-2.57

SHT. NO.	STATION TO STATION	203 EXCAVATION NOT INCLUD. EMBANKMENT CONSTRUCTION CU.YD	203 EMBANKMENT CU.YD	870 SEEDING AND MULCHING SQ.YD
	PROPOSED USR 35			
54	866+00 BACK TO 142+00 AHEAD	4414	14823	13713
55	142+00 TO 156+00	12401	7760	31820
56	156+00 TO 170+00	114008	151	31210
57	170+00 TO 184+00	7055	35059	32917
58	184+00 TO 198+00	836	194492	32484
59	198+00 TO 212+00	1292	200859	40329
60	212+00 TO 226+00	869	89948	35627
61	226+00 TO 240+00	8802	22560	30693
62	240+00 TO 254+00	10992	56152	28858
63	254+00 TO 268+00	5446	58332	34023
64	268+00 TO 282+00	1878	30807	30387
65	282+00 TO 296+00	20219	3318	32689
66	296+00 TO 310+00	8024	39525	37494
67	310+00 TO 324+00	5383	31626	31806
68	324+00 TO 338+00	3027	97247	34884
69	338+00 TO 352+00	13015	155494	46971
70	352+00 TO 366+00	14739	44136	43280
71	366+00 TO 380+00	29660	3515	37966
72	380+00 TO 394+00	46039	2015	34527
73	394+00 BACK TO 380+00 AHEAD	247	2139	5204
	CR-16			
162	16+00 TO 30+00	1050	10221	9517
163	30+00 TO 40+00	750	10591	9629
	TR-94			
174	0+00 TO 5+00	210	3196	3507
	I-71			
177	395+00 TO 404+00	0	11854	4020
183				
184	EXIST.-USR35			
185	60+00 TO 73+00	1972	38542	16774
186	73+00 TO 86+00	297	93224	16209
	86+00 TO 99+00	1844	4370	13409
207	99+00 TO 104+00	285	21	2370
208				
	RAMP C			
	227+50 TO 234+00	4	51428	10049
216	234+00 TO 241+25	1687	8404	8695
	RAMP D			
	237+25 TO 248+00	1179	18383	17727
222	SERVICE ROAD 1	78	1312	3198
222	SERVICE ROAD 2	24	991	1253
228	SERVICE ROAD DRIVE	136	63	300
	SERVICE ROAD 3			
229	13+00 TO 16+50			
230	16+50 TO 30+00	245	567	5937
		2021	5215	17926
	SR 729			
237	152+75 TO 163+00	766	42989	17583
238	163+00 TO 175+00	494	59730	20065
239	175+00 TO 181+44	1592	5113	7652
326	CHANNEL NO. 1	3037	0	3025
329	CHANNEL NO. 2	5120	0	5148
	*ADDITIONAL SEEDING & MULCHING			45111
	*ADDITIONAL EMBANKMENT		19500	
	SUB-TOTAL	**330,117**	**1,475,672**	**885,986**
	DEDUCT FOR ITEM 660			-1098
	DEDUCT FOR ITEM 667			-6239
	DEDUCT FOR ITEM 670			-60129
	*DEDUCT EXCAVATION	-5255		
	TOTAL TO SHEET NO. 33	325,882	**1,475,672**	**818,520**

R/W SHT. NO.	STATION TO STATION	607 FENCE, TYPE 47 LIN. FT.
495	866+00 TO 142+00	1294
496	142+00 TO 156+00	2800
497	156+00 TO 170+00	2832
498	170+00 TO 184+00	2802
499	184+00 TO 198+00	2666
500	198+00 TO 212+00	2811
501	212+00 TO 226+00	2806
502	226+00 TO 240+00	2389
503	240+00 TO 254+00	2711
504	254+00 TO 268+00	2801
505	268+00 TO 282+00	2805
506	282+00 TO 296+00	2800
507	296+00 TO 310+00	2780
508	310+00 TO 324+00	2752
509	324+00 TO 338+00	2801
510	338+00 TO 352+00	3307
511	352+00 TO 366+00	2800
512	366+00 TO 380+00	2798
513	380+00 TO 392+00	2400
514	392+00 TO 386+00 AHEAD	784
517	60+00 TO 73+00 EX. 35	1338
518	86+00 TO 96+00 EX. 35	1771
	TOTAL TO SHEET NO. 33	55048

ITEM 605- AGGREGATE DRAINS

C.R. - STA. 20+25 TO STA. 28+00
32 DRAINS @ 13 L.F.(AVG) = 416 L.F.
STA. 31+75 TO STA. 38+75
28 DRAINS @ 13 L.F.(AVG) = 364 L.F.

T.R.94 - STA. 0+50 TO 3+75
14 DRAINS @ 13 L.F.(AVG) = 182 L.F.

S.R.729 - STA. 152+75 TO STA. 165+25
50 DRAINS @ 13 L.F.(AVG) = 650 L.F.
STA. 168+75 TO STA. 181+25
50 DRAINS @ 13 L.F.(AVG) = 650 L.F.

SERVICE ROAD NO. 1
STA. 10+50 TO STA. 13+00
9 DRAINS @ 14 L.F.(AVG) = 126 L.F.

SERVICE ROAD NO. 2
STA. 8+00 TO STA. 9+50
9 DRAINS @ 14 L.F.(AVG) = 98 L.F.

SERVICE ROAD NO. 3
STA. 16+75 TO STA. 29+75
52 DRAINS @ 14 L.F.(AVG) = 728 L.F.

TOTAL = 3214 L.F.

ITEM 870 - COMMERCIAL FERTILIZER △

(818,639 SY)(9 SF/SY)(30#/1000 SF)(1 TON/2000#) = 110.50 TONS
(USE 110 TONS)

ITEM 870 - AGRICULTURAL LIMING △

(818,639 SY)(9 SF/SY)(92#/1000 SF)(220%)(1 TON/2000#) = 745.80 TONS
(USE 746 TONS)

ITEM 203 - PROOF ROLLING

275,553 SY ÷ 3000 SY/HR = 91.90 HRS (USE 92 HRS)

ITEM 604 - REFERENCE MONUMENTS

FROM SHEET NO. 483	15 EACH
FROM SHEET NO. 484	14 EACH
TOTAL	29 EACH

ITEM 604 - MONUMENT ASSEMBLY

FROM SHEET NO. 483	11 EACH
FROM SHEET NO. 484	6 EACH
TOTAL	17 EACH

△ INCLUDES 119 SQ.YDS. FROM SHEET 39

* CHANGES PER SHOULDER REVISION
SEE NOTE ON SHEET 12A

REVISED 1/99 BY FYC

ESTIMATING QUANTITIES

FAY-35-2.57

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&75\40lnewsht\new042.dgn 20-MAR-2001 13:36PM dwise
i:\pr\project\fay35\fay-35-2.57\fay42gb.dgn 01-FEB-2000 2:52PM fcheek
i:\pr\project\fay35\fay-35-2.57\fay42gb.dgn

PAVEMENT SUB - SUMMARY

STATION TO STATION	203 SUBGRADE COMPACTION	301 BITUMINOUS AGGREGATE BASE, PG64-22	304 AGGREGATE BASE	307 NON-STABILIZED DRAINAGE BASE, TYPE IA	408 BITUMINOUS PRIME COAT (0.4 Gal./S.Y.)	446 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22	446 ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22	448 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22	611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (t=13")	611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (t=15")	407 TACK COAT FOR INTERMEDIATE COURSE (0.010 GAL/SY)	880 ASPHALT CONCRETE (7 YEAR WARRANTY)	448 1¾ ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22	411 8" STABILIZED CRUSHED AGGREGATE
	SQ.YD.	CU.YD.	CU.YD.	SQ.YD.	GAL.	CU.YD.	CU.YD.	CU.YD.	SQ.YD.	SQ.YD.	GAL.	CU.YD.	CU.YD.	CU.YD.
PROPOSED USR 35														
135 + 72.80 TO 137 + 19.28	1324.0		235.7	1321.6	480.9							537.0		
137 + 19.28 TO 137 + 39.28	244.4		42.2						244.4					
138 + 60.72 TO 138 + 80.72	244.4		42.2						244.4					
138 + 80.72 TO 193 + 30.11	48463.1		8447.8	48460.4	17611.3							11023.4		
193 + 30.11 TO 193 + 50.11	244.4		42.2						244.4					
196 + 16.13 TO 196 + 36.13	244.4		42.2						244.4					
196 + 36.13 TO 238 + 50.00	37693.2		6569.4	37666.9	13695.9							8562.1		
238 + 50.00 TO 340 + 44.92	90642.5		15995.9	90640.2	33142.0							31366.60		
340 + 44.92 TO 340 + 69.92	305.6		52.8							305.6				
342 + 24.96 TO 342 + 49.96	305.6		52.8							305.6				
342 + 49.96 TO 396 + 20.24	47756.8		8455.5	47754.5	17354.7							16561.3		
C R 16														
20 + 00.00 TO 28 + 45.15	2077.4	237.6	504.2		831.0			72.2						
28 + 45.15 TO 28 + 65.15	82.2		14.4						82.2					
31 + 34.85 TO 31 + 54.85	82.2		14.4						82.2					
31 + 54.85 TO 39 + 00.00	1835.0	209.8	439.2		734.0			63.7						
T R 94														
0 + 11.00 TO 4 + 00.00	974.9	111.7	244.0		390.0			33.9						
EXISTING USR 35														
62 + 25.00 TO 71 + 97.56	8073.0	1818.1	1387.6		3229.2	280.3	392.4				80.7			
71 + 97.56 TO 75 + 84.13	2420.6	546.2	420.1		967.0	84.0	117.5				24.2			
75 + 84.13 TO 76 + 09.13	175.0		30.1							175.0				
78 + 61.13 TO 78 + 86.13	175.0		30.1							175.0				
78 + 86.13 TO 90 + 00.00	6775.0	1531.6	1176.2		2710.0	235.2	329.3				67.8			
SERVICE ROAD 1														
10 + 34.52 TO 13 + 00.00	768.3	65.3	175.9		307.3			26.7						
SERVICE ROAD 2														
8 + 00.00 TO 9 + 58.83	478.7	40.7	113.4		191.5			16.6						
SERVICE ROAD 3														
15 + 50.00 TO 29 + 88.00	3961.5	350.5	967.5		1584.6			137.6						
RAMP C														
227 + 48.46 TO 230 + 59.33	1017.4		305.8		356.7							357.7		
230 + 59.33 TO 237 + 19.33	2639.7		782.8		948.5							919.5		
237 + 19.33 TO 241 + 20.14	1113.4		339.4		380.2							395.2		
241+20.14 TO 256+07.25 ACCELERATION LANE	2593.1		467.2	2593.1	914.1							909.8		
256 + 07.25 TO 258 + 00.00	235.6		44.3	235.6								80.8		
RAMP CC														
229 + 13.33 TO 230 + 10.32	279.0	75.4	84.5		96.4	9.7	13.6				2.8			
RAMP D														
237 + 22.03 TO 238 + 00.00	340.6		100.8		122.9							118.5		
238 + 00.00 TO 248 + 47.74	2929.5		892.7		1000.6							1039.7		
248+47.74 TO 256+38.05 DECELERATION LANE	1108.6		202.9	1108.6	379.8							393.1		
SR 729														
152 + 75.00 TO 165 + 53.08	3144.0	359.4	779.9		1257.6	109.2	152.8				31.4			
165 + 53.08 TO 165 + 73.08	82.2		14.4						82.2					
168 + 25.18 TO 168 + 45.18	82.2		14.4						82.2					
168 + 45.18 TO 181 + 44.00	3213.3	367.3	808.0		1285.3	111.6	156.2				32.1			
U-TURN MEDIAN OPENING														
280 + 28	94.9	26.1	30.7		38.0							34.0		
382 + 53	94.9	26.1	30.7		38.0							34.0		
HARMONY ROAD														
24+50 TO 35.63	204.2		51.6		87.9			7.6			22.0		10.7	8.4
25+63 TO 29+24	802.2		187.2		320.9			27.9			80.2		39.0	71.3
29+24 TO 30+44	231.1		54.3		92.5			8.0			23.1		11.2	9.0
SUBTOTAL	275553.1	5765.8	50687.4	229780.9	100548.8	830.0	1161.8	394.2	1306.4	961.2	364.3	72332.7	60.9	88.7
TOTALS TO GENERAL SUMMARY	275553	5766	50687	229781	100549	830	1162	394	1306	962	364	72333	61	89

REVISED 1/00 BY FYC
CALCULATED M.T.
CALCULATED W.G.Y.

PAVEMENT ESTIMATED QUANTITIES

PAVEMENT

FAY-35-2.57

CONSTANTS MAINLINE

880 EDGE COURSE

WITH 14" COURSE 0.92' X 1.25' X 1' / 27 CY/CF = 0.0426 CY/LF
WITH 12" COURSE 0.75' X 1.08' X 1' / 27 CY/CF = 0.03 CY/LF
WITH 8" COURSE 0.42' X 0.75' X 1' / 27 CY/CF = 0.0117 CY/LF

304 EDGE COURSE

WITH 14" 880 COURSE 1.42' X 0.5' X 1' / 27 CY/CF = 0.0263 CY/LF
WITH 12" 880 COURSE 1.25' X 0.5' X 1' / 27 CY/CF = 0.0231 CY/LF
WITH 8" 880 COURSE 0.92' X 0.5' X 1' / 27 CY/CF = 0.0170 CY/LF
ON RAMPS (0.83x0.92+0.5x0.5) x 1 / 27 CY/CF = 0.0375 CY/LF

PROPOSED USR 35

STA 135+72.80 TO STA 137+19.28

AREA = 146.48 LF X 2 SIDES X 40' / 9 SF/SY = 1302.0 SY
ADDITIONAL SHOULDER AREA AT CURBS:
STA 136+60.31 TO STA 136+80.31 LT = 20 LF X 4' / 9 SF/SY = 8.9 SY
STA 137+34.25 TO STA 137+58.25 RT = 24 LF X 4' / 9 SF/SY = 10.7 SY
OUTSIDE EDGE COURSES:
STA 135+72.80 TO STA 136+60.31 LT = 87.51 LF
STA 135+72.80 TO STA 137+34.25 RT = 161.45 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)

(1302.0 SY + 8.9 SY + 10.7 SY) X 14" / 36"/YD = 513.9 CY
ADD MEDIAN EDGE COURSES:
(2 SIDES X 146.48 LF X 0.0426) CY/LF = 12.5 CY
ADD OUTSIDE EDGE COURSES:
(87.51 LF + 161.45 LF) X 0.0426 CY/LF = 10.6 CY
537.0 CY

ITEM 307 - NON STABILIZED DRAINAGE BASE, TYPE IA

(1302.0 SY + 8.9 SY + 10.7 SY) = **1321.6 SY**

ITEM 408 - BITUMINOUS PRIME COAT

(1302.0 SY + 8.9 SY + 10.7 SY) X 0.4 GAL/SY = 528.6 GAL
DEDUCT FOR UNDERDRAINS:
(146.48 LF X 1.83' X 4 SIDES/9 SF/SY) X 0.4 GAL/SY = -47.7 GAL
480.9 GAL

ITEM 304 - AGGREGATE BASE

(1302.0 SY +8.9 SY + 10.7 SY) X 6" / 36"/YD = 220.3 CY
ADD EDGE COURSES:
MEDIAN: 146.48 LF X 0.0263 CY/LF X 2 SIDES = 7.7 CY
OUTSIDE SHOULDERS:
(87.51 LF + 161.45 LF) X 0.0263 CY/LF = 6.5 CY
AT CURBS:
(20' LF + 24' LF) X 1.5' X 0.5' / 27 CY/CF = 1.2 CY
235.7 CY

ITEM 203 - SUBGRADE COMPACTION

PAVEMENT AND SHOULDERS: 1302.0 SY
ADD ADDITIONAL SHOULDER AT CURBS:
8.9 SY + 10.7 SY = 19.6 SY
ADD UNDER CURBS:
(20' LF + 24' LF) X 0.5' / 9 SF/SY = 2.4 SY
1324.0 SY

STA 137+19.28 TO STA 137+39.28

AREA = 2 SIDES X 20 LF X 55' / 9 SF/SY = 244.4 SY
EDGE COURSES: 4 SIDES X 20 LF X 1' / 9 SF/SY = 8.9 SY

ITEM 611 - REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (T=13") = 244.4 SY

ITEM 304 - AGGREGATE BASE

(244.4 CY + 8.9 SY) X 6" / 36"/YD = **42.2 CY**

ITEM 203 - SUBGRADE COMPACTION = 244.4 SY

STA 138+60.72 TO STA 138+80.72

AREA = 2 SIDES X 20 LF X 55' / 9 SF/SY = 244.4 SY
EDGE COURSES: 4 SIDES X 20 LF X 1' / 9 SF/SY = 8.9 SY

ITEM 611 - REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (T=13") = 244.4 SY

ITEM 304 - AGGREGATE BASE

(244.4 SY + 8.9 SY) X 6" / 36"/YD = **42.2 CY**

ITEM 203 SUBGRADE COMPACTION = 244.4 SY

STA 138+80.72 TO STA 193+30.11

AREA 5449.39 LF X 2 SIDES X 40' / 9 SF/SY = 48439.0 SY
ADDITIONAL SHOULDER AREA AT CURBS:
STA 193+06.80 TO STA 193+30.44 LT = 24 LF X 4' / 9 SF/SY = 10.7 SY
STA 193+04.76 TO STA 193+29.13 RT = 24 LF X 4' / 9 SF/SY = 10.7 SY
OUTSIDE EDGE COURSES:
STA 138+80.72 TO STA 173+75.49 LT & RT = 3494.77 LF
STA 173+75.49 TO STA 193+06.80 LT= 1931.31 LF X 1.0140 = 1958.35 LF
STA 173+75.49 TO STA 193+04.76 RT = 1929.27 LF X 0.9860 = 1902.26 LF

ITEM 880 - ASPHALT CONCRETE (7 YEAR WARRANTY)

(48439.0 SY + 10.7 SY + 10.7 SY) X 8" / 36"/YD = 10769.0 CY
ADD MEDIAN EDGE COURSES;
2 SIDES X 5449.39 LF X 0.0117 CY/LF = 127.5 CY
ADD OUTSIDE EDGE COURSES: ((3494.77 LF x 2 SIDES) + 1958.35 LF + 1902.26 LF) X 0.0117 CY/LF = 126.9 CY
11023.4 CY

ITEM 307 - NON STABILIZED DRAINAGE BASE, TYPE IA

(48439.0 SY + 10.7 SY + 10.7 SY) = **48460.4 SY**

ITEM 408 - BITUMINOUS PRIME COAT

(48439.0 SY + 10.7 SY + 10.7 SY) X 0.4 GAL/SY = 19384.2 GAL
DEDUCT FOR UNDERDRAINS: (5449.39 LF X 1.83' X 4 SIDES / 9 SF/SY) X 0.4 GAL/SY = -1772.9 GAL
17611.3 GAL

ITEM 304 - AGGREGATE BASE

(48439.0 SY + 10.7 SY + 10.7 SY) X 6" / 36"/YD = 8076.7 CY
ADD EDGE COURSES:
MEDIAN: 5449.39 LF X 0.0170 CY/LF X 2 SIDES = 185.3 CY
OUTSIDE SHOULDERS: (3494.77 LF X 2 SIDES + 1958.35 LF + 1902.26 LF) X 0.0170 = 184.5 CY
AT CURBS: (24 LF+24 LF) X 1.5' X 0.5' /27 CY/CF = 1.3 CY
8447.8 CY

ITEM 203 - SUBGRADE COMPACTION

PAVEMENT AND SHOULDERS: 48439.0 SY
ADD ADDITIONAL SHOULDER AT CURBS:
(10.7 SY + 10.7 SY) = 21.4 SY
ADD UNDER CURBS: (24 LF + 24 LF) X 0.5' /9 SF/SY = 2.7 SY
48463.1 SY

STA 193+30.11 TO STA 193+50.11

AREA = 2 SIDES X 20 LF X 55' / 9 SF/SY = 244.4 SY
EDGE COURSES: 4 SIDES X 20 LF X 1' / 9 SF/SY = 8.9 SY

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (T=13") 244.4 SY

ITEM 304 AGGREGATE BASE

(244.4 SY + 8.9 SY) X 6" / 36"/YD = **42.2 CY**

ITEM 203 SUBGRADE COMPACTION = 244.4 SY

STA 196+16.13 TO STA 196+36.13

AREA = 2 SIDES X 20 LF X 55' / 9 SF/SY = 244.4 SY
EDGE COURSES: 4 SIDES X 20 LF X 1' / 9 SF/SY = 8.9 SY

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN (T=13") = 244.4 SY

ITEM 304 AGGREGATE BASE

(244.4 SY + 8.9 SY) X 6" / 36"/YD = **42.2 CY**

ITEM 203 SUBGRADE COMPACTION = 244.4 SY

STA 196+36.13 TO STA 238+50

AREA = 4213.87 LF X 2 SIDES X 40' / 9 SF/SY = 37456.6 SY
ADDITIONAL SHOULDER AREA AT CURBS:
STA 196+34.25 TO STA 196+57.80 LT = 24 LF X 4' / 9 SF/SY = 10.7 SY
STA 196+38.17 TO STA 200+93.98 RT = 449 LF X 4' / 9 SF/SY = 199.6 SY
OUTSIDE EDGE COURSES:
STA 196+57.80 TO PT STA 216+38.09 LT = 1980.29 LF X 1.0140 = 2008.01 LF
STA 200+93.98 TO PT STA 216+38.09 RT = 1544.11 LF X 0.9860 = 1522.49 LF
STA 216+38.09 TO STA 238+50 LT & RT = 2211.91 LF

ITEM 880 - ASPHALT CONCRETE (7 YEAR WARRANTY)

(37456.6 SY + 10.7 SY + 199.6 SY) X 8" / 36"/YD = 8370.4 CY
ADD MEDIAN EDGE COURSES:
4213.87 LF X 2 SIDES X 0.0117 CY/LF = 98.6 CY
ADD OUTSIDE EDGE COURSES: (2008.1 LF + 1522.49 LF + (2211.91 LF X 2 SIDES)) X 0.0117 CY/LF = 93.1 CY
8562.1 CY

ITEM 307 - NON-STABILIZED DRAINAGE BASE, TYPE IA

(37456.6 SY + 10.7 SY + 199.6 SY) = **37666.9 SY**

ITEM 408 - BITUMINOUS PRIME COAT

(37456.6 SY +10.7 SY +199.6 SY) X 0.4 GAL/SY = 15066.8 GAL
DEDUCT FOR UNDERDRAINS: 4213.87 LF X 1.83' X 4 SIDES /9 SF/SY) X 0.4 GAL/SY = -1370.9 GAL
13695.9 GAL

ITEM 304 - AGGREGATE BASE

37456.6 SY+ 10.7 SY + 199.6 SY) X 6" / 36"/YD = 6277.8 CY
ADD EDGE COURSES:
MEDIAN: 4213.87 LF X 0.0170 CY/LF X 2 SIDES = 143.3 CY
OUTSIDE SHOULDERS: (2008.01 LF + 1522.49 LF + 2211.91 LF X 2 SIDES X 0.0170 CY/LF = 135.2 CY
AT CURBS: (24 LF + 449 LF) X 1.5' X 0.5'/ 27 CY/CF = 13.1 CY
6569.4 CY

ITEM 203 - SUBGRADE COMPACTION

PAVEMENT AND SHOULDERS: 37456.6 SY
ADD ADDITIONAL SHOULDERS AT CURBS:
(10.7 SY + 199.6 SY) = 210.3 SY
ADD UNDER CURBS:
(24 LF +449 LF) X 0.5' /9 SF/SY = 26.3 SY
37693.2 SY

STA 238+50 TO STA 340+44.92

AREA = 10194.92 LF X 2 SIDES X 40' / 9 SF/SY = 90621.5 SY
ADDITIONAL SHOULDER AREA AT CURBS:
STA 339+59.18 TO STA 339+83.18 RT = 24' X 4' /9 SF/SY = 10.7 SY
STA 340+89.12 TO STA 341+07.12 LT= 18' X 4' /9 SF/SY = 8.0 SY
OUTSIDE EDGE COURSES:
10194.92 LF X 4 SIDES = 40779.68 LF
DEDUCTIONS:
AT RAMP C STA 241+23.08 TO STA 256+07.25
(1484.17' X 1.0164) = -1508.51 LF
AT RAMP D STA 248+38.05 TO STA 256+38.05
(800.0' X 0.9836) = -786.88 LF
AT CURBS (24 LF + 18 LF) = -42.00 LF
38442.29 LF

ITEM 880 - ASPHALT CONCRETE (7 YEAR WARRANTY)

(90621.5 SY + 10.7 SY + 8.0 SY) X 12" / 36"/YD = 30213.3 CY
ADD EDGE COURSES: 38442.29 LF X 0.03 CY/LF = 1153.3 CY
31366.6 CY

ITEM 307 - NON-STABILIZED DRAINAGE BASE, TYPE IA

(90621.5 SY + 10.7 SY + 8.0 SY) = **90640.2 SY**

ITEM 408 - BITUMINOUS PRIME COAT

(90621.5 SY + 10.7 SY + 8.0 SY) X 0.4 GAL/SY = 36256.1 GAL
DEDUCT FOR UNDERDRAINS: (10194.92 LF X 1.83' X 4 SIDES/9 SF/SY) X 0.4 GAL/SY = -3316.7 GAL
ADD AT RAMPS: [(1704.08' + 786.88') X 1.83'/ 9 SF/SY] X 0.4 GAL/SY = 202.6 GAL
33142.0 GAL

ITEM 304 - AGGREGATE BASE

(90621.5 SY + 10.7 SY + 8.0 SY) X 6" / 36"/YD = 15106.7 CY
ADD EDGE COURSES: 38442.29 LF X 0.0231 CY/LF = 888.0 CY
ADD AT CURBS: (24 LF + 18 LF) X 1.5' X 0.5' / 27 CY/DF = 1.2 CY
15995.9 CY

CALCULATED M.T. CHECKED W.G.Y.
REVISED 10-99 BY GFR

PAVEMENT CALCULATIONS

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\fay44qc.dgn 01-FEB-2000 2:51PM fcheek
u:\pr\project\fay35\fay-35-2.57\ fay44go.dgn

PROPOSED U.S. 35, CON'T.

ITEM 203 SUBGRADE COMPACTION

PAVEMENT AND SHOULDERS = 90621.5 SY
ADD ADDITIONAL SHOULDER AT CURBS: (10.7 SY +8.0 SY) = 18.7 SY
ADD UNDER CURBS: (24 LF + 18 LF) X 0.5′ / 9 SF/SY = 2.3 SY
90642.5 SY

STA 340+44.92 TO STA 340+69.92

AREA = 2 SIDES X 25′ X 55′ / 9 SF/SY = 305.6 SY
EDGE COURSES: 4 SIDES X 25 LF X 1′ / 9 SF/SY = 11.1 SY

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=15") = 305.6 SY

ITEM 304 AGGREGATE BASE
(305.6 SY + 11.1 SY) X 6″ / 36″/YD = **52.8 CY**

ITEM 203 SUBGRADE COMPACTION = 305.6 SY

STA 342+24.96 TO STA 342+49.96

AREA = 2 SIDES X 25′ X 55′ / 9 SF/SY = 305.6 SY
EDGE COURSE: 4 SIDES X 25 LF X 1′ / 9 SF/SY = 11.1 SY

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=15") = 305.6 SY

ITEM 304 AGGREGATE BASE
(305.6 SY + 11.1 SY) X 6″ / 36″/YD = **52.8 CY**

ITEM 203 SUBGRADE COMPACTION = 305.6 SY

STA 342+49.96 TO STA 396+20.24

AREA = 5370.28 LF X 2 SIDES X 40′ / 9 SF/SY = 47735.8 SY
ADDITIONAL SHOULDER AREA AT CURBS:
STA 341+87.76 TO STA 342+05.76 RT= 18 LF X 4′ /9 SF/SY= 8.0 SY
STA 343+11.70 TO STA 343+35.70 LT= 24 LF X 4′ /9 SF/SY= 10.7 SY
OUTSIDE EDGE COURSES:
5370.28 LF X 4 SIDES - CURBS (18 LF + 24 LF) = 21439.12 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
(47735.8 SY + 8.0 SY + 10.7 SY) X 12″ / 36″/YD = 15918.1 CY
ADD EDGE COURSES: 21439.12 LF X 0.03 CY/LF = 643.2 CY
16561.3 CY

ITEM 307 NON-STABILIZED DRAINAGE BASE, TYPE IA
(47735.8 SY + 8.0 SY + 10.7 SY) = **47754.5 SY**

ITEM 408 BITUMINOUS PRIME COAT
(47735.8 SY + 8.0 SY + 10.7 SY) X 0.4 GAL/SY = 19101.8 GAL
DEDUCT FOR UNDERDRAINS:
(5370.28 LF X 1.83′ X 4 SIDES /9 SF/SY) X 0.4 GAL/SY = -1747.1 GAL
17354.7 GAL

ITEM 304 AGGREGATE BASE
(47735.8 SY + 8.0 SY + 10.7 SY) X 6″ / 36″/YD = 7959.1 CY
ADD EDGE COURSES: 21439.12 LF X 0.0231 CY/LF = 495.2 CY
ADD AT CURBS: (18 LF + 24 LF) X 1.5′X 0.5′/27 CF/CY = 1.2 CY
8455.5 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AND SHOULDERS: 47735.8 SY
ADD ADDITIONAL SHOULDER AT CURBS: (8.0 SY+10.7 SY) = 18.7 SY
ADD UNDER CURBS: (18 LF + 24 LF) X 0.5′ /9 SF/SY = 2.3 SY
47756.8 SY

COUNTY ROAD 16

STA 20+00 TO STA 28+45.15

PAVEMENT AREA:
STA 20+00 TO STA 21+37.50
1/2(19.5′ + 22.0′) X 137.5 LF / 9 SF/SY = 317.0 SY
STA 21+37.5 TO STA 28+45.15
22.0′ X 707.65 LF / 9 SF/SY = 1729.8 SY
ADDITIONAL AREA AT CURBS:
STA 28+13.25 TO STA 28+36.25 RT
23.0 LF X 6′ / 9 SF/SY = 15.3 SY
STA 28+31.32 TO STA 28+54.32 LT
23.0 LF X 6′ / 9 SF/SY = 15.3 SY
2077.4 SY

SHOULDER AREA:
STA 20+00 TO STA 20+50 =
1/2(2′ + 4′) X 50 LF X 2 SIDES / 9 SF/SY = 33.3 SY
STA 20+50 TO STA 28+13.25: RT=763.25 LF X 4′ /9 SF/SY= 339.2 SY
STA 20+50 TO STA 28+31.32: LT=781.32 LF X 4′ /9 SF/SY= 347.3 SY
DEDUCT AREA IN DRIVE I-DW (COMPUTER) = -34.3 SY
685.5 SY

EDGE COURSE LENGTH
(137.5 LF + 707.65 LF) X 2 SIDES = 1690.3 LF
DEDUCT AT CURBS: 23′ + 23′ = - 46.0 LF
1644.3 LF

ITEM 448 ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
2077.4 SY X 1.25″ / 36″/YD = **72.2 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
2077.4 SY X 4″ / 36″/YD = 230.8 CY
ADD EDGE COURSE:
1644.3 LF X 0.333′ X 0.333′ / 27 CF/CY = 6.8 CY
237.6 CY

ITEM 408 BITUMINOUS PRIME COAT
2077.4 SY X 0.4 GAL/SY = **831.0 GAL**

ITEM 304 AGGREGATE BASE
PAVEMENT: 2077.4 SY X 6″ / 36″/YD = 346.2 CY
EDGE COURSE:
1644.3 LF X 0.5′ X 0.83′ / 27 CF/CY = 25.3 CY
SHOULDER: 685.5 SY X 8″ / 36″/YD = 152.3 CY
DEDUCT AREA IN EDGE COURSE:
1644.3 LF X 0.322 SF / 27 CF/CY = - 19.6 CY
504.2 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **2077.4 SY**

STA 28+45.15 TO STA 28+65.15

AREA: 20 LF X 37′ / 9 SF/SY = 82.2 SY
EDGE COURSE: 20 LF X 2 SIDES = 40.0 LF

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=13") 82.2 SY

ITEM 304 AGGREGATE BASE
82.2 SY X 6″ / 36″/YD = 13.7 CY
EDGE COURSE: 40 LF X 1′ X 0.5′ / 27 CF/CY = 0.7 CY
14.4 CY

ITEM 203 SUBGRADE COMPACTION = 82.2 SY

STA 31+34.85 TO STA 31+54.85

AREA: 20 LF X 37′ / 9 SF/SY = 82.2 SY
EDGE COURSE: 20 LF X 2 SIDES = 40.0 LF

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=13") = 82.2 SY

ITEM 304 AGGREGATE BASE
82.2 SY X 6″ / 36″/YD = 13.7 CY
EDGE COURSE 40 LF X 1′ X 0.5′ / 27 CF/CY = 0.7 CY
14.4 CY

ITEM 203 SUBGRADE COMPACTION = 82.2 SY

STA 31+54.85 TO STA 39+00

PAVEMENT AREA:
STA 31+54.85 TO STA 37+62.50 =
607.65 LF X 22′ / 9 SF/SY = 1485.4 SY
STA 37+62.50 TO STA 39+00 LT =
1/2(11′ + 8.5′) X 137.50′ / 9 SF/SY = 149.0 SY
STA 37+62.5 TO STA 38+50 RT =
11′ X 87.5′ / 9 SF/SY = 106.9 SY
STA 38+50 TO STA 39+00 RT =
1/2(11′ +011.7′) X 50′ / 9 SF/SY = 63.1 SY
ADDITIONAL AREA AT CURBS:
STA 31+45.68 TO STA 31+68.68 RT =
23 LF X 6′ / 9 SF/SY = 15.3 SY
STA 31+63.75 TO STA 31+86.75 LT =
23 LF X 6′ / 9 SF/SY = 15.3 SY
1835.0 SY

SHOULDER AREA:
STA 31+68.68 TO STA 33+02.74 RT
134.06 LF X 4′ / 9 SF/SY = 59.6 SY
STA 34+22.71 TO STA 38+50.00 RT
427.29 LF X 4′ / 9 SF/SY = 189.9 SY
STA 38+50.00 TO STA 39+00.00 RT
50 LF X 1/2(4′ + 2′) / 9 SF/SY = 16.7 SY
STA 31+86.75 TO STA 38+50 LT
663.25 LF X 4′ / 9 SF/SY = 294.8 SY
STA 38+50 TO STA 39+00 LT
50 LF X 1/2(4′ + 2′) / 9 SF/SY = 16.7 SY
577.7 SY

EDGE COURSE LENGTH:
(607.65 LF + 137.50 LF) X 2 SIDES = 1490.3 LF
DEDUCT AT CURBS (23′ + 23′) = - 46.0 LF
1444.4 LF

ITEM 448 ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
1835.0 SY X 1.25″ / 36″/YD = **63.7 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
1835.0 SY X 4″ / 36″/YD = 203.9 CY
ADD EDGE COURSE:
1444.4 LF X 0.333′ X 0.333′ / 27 CF/CY = 5.9 CY
209.8 CY

ITEM 408 BITUMINOUS PRIME COAT
1835.0 SY X 0.4 GAL/SY = **734.0 GAL**

ITEM 304 AGGREGATE BASE
PAVEMENT: 1835.0 SY X 6″ / 36″/YD = 305.8 CY
EDGE COURSE:
1444.4 LF X 0.5 X 0.83′ / 27 CF/CY = 22.2 CY
SHOULDER: 577.7 SY X 8″ / 36″/YD = 128.4 CY
DEDUCT AREA IN EDGE COURSE:
1444.4 LF X 0.322 SF / 27 CF/CY = - 17.2 CY
439.2 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **1835.0 SY**

TOWNSHIP ROAD - 94

STA 0+11 TO STA 4+00

PAVEMENT AREA:
STA 0+11.00 TO STA 3+34.00 =
323 LF X 20′ / 9 SF/SY = 717.8 SY
STA 3+34.00 TO STA 4+00.00 =
1/2(20 + 17.6) X 66 LF / 9 SF/SY = 137.9 SY
ADD NE RETURN AREA (COMPUTER) 56.6 SY
ADD SE RETURN AREA (COMPUTER) 62.6 SY
974.9 SY

EDGE COURSE:
NE RETURN (COMPUTER) = 77.44 LF
STA 0+59.69 TO STA 4+00 LT = 340.31 LF
SE RETURN (COMPUTER) = 79.60 LF
STA 0+62.28 TO STA 4+00 RT = 337.72 LF
835.07 LF

SHOULDERS:
NE RETURN (COMPUTER) = 33.0 SY
SE RETURN (COMPUTER) = 34.0 SY
STA 0+59.69 TO STA 3+34.00 LT =
274.31′ X 4′ / 9 SF/SY = 121.9 SY
STA 3+34.00 TO STA 4+00 LT =
66 LF X 1/2(4′ + 2′) / 9 SF/SY = 22.0 SY
STA 0+62.28 TO STA 3+34.00 RT =
271.72′ X 4′ / 9 SF/SY = 120.8 SY
STA 3+34 TO STA 4+00 RT =
66 LF X 1/2(4′ + 2′) / 9 SF/SY = 22.0 SY
353.7 SY

ITEM 448 ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
974.9 SY X 1.25″ / 36″/YD = **33.9 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
974.9 SY X 4″ / 36″/YD = 108.3 CY
ADD EDGE COURSE:
835.07 LF X 0.333′ X 0.333′ / 27 CF/CY = 3.4 CY
111.7 CY

ITEM 408 BITUMINOUS PRIME COAT
974.9 SY X 0.4 GAL/SY = **390.0 GAL**

ITEM 304 AGGREGATE BASE
PAVEMENT: 974.9 SY X 6″ / 36″/YD = 162.5 CY
EDGE COURSE:
835.07 LF X 0.5′ X 0.83′ / 27 CF/CY = 12.9 CY
SHOULDERS: 353.7 SY X 8″ / 36″/YD = 78.6 CY
DEDUCT AREA IN EDGE COURSE:
835.07 LF X 0.322 SF / 27 CF/CY = - 10.0 CY
244.0 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **974.9 SY**

CALCULATED M.T. CHECKED W.G.Y.
REVISED 10-99 BY GFR

PAVEMENT CALCULATIONS

FAY-35-2.57

EX. USR 35

STA 62+25 TO STA 71+97.56
AREA: STA 62+25 TO STA 68+85 =
660 LF X 1/2 (83 + 68') / 9 SF/SY = 5536.7 SY
STA 68+85 TO STA 69+87.48 =
102.48' X 68' / 9 SF/SY = 774.3 SY
STA 69+87.48 TO STA 71+97.56 (COMPUTER AREA) = 1762.0 SY
8073.0 SY

EDGE COURSE: LEFT = 972.73 LF
RIGHT = 968.56 LF
1941.29 LF

ITEM 446 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
8073.0 SY X 1.25" / 36"/YD = **280.3 CY**

ITEM 446 ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
8073.0 SY X 1.75" / 36"/YD = **392.4 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
8073.0 SY X 8" / 36"/YD = 1794.0 CY
EDGE COURSE
1941.29 LF X 0.67' X 0.5' AVG / 27 CF/CY = 24.1 CY
1818.1 CY

ITEM 408 BITUMINOUS PRIME COAT
8073.0 SY X 0.4 GAL/SY = **3229.2 GAL**

ITEM 304 AGGREGATE BASE
8073.0 SY X 6" / 36"/YD = 1345.5 CY
EDGE COURSE
1941.29 LF X 0.5' 1.17' / 27 CF/CY = 42.1 CY
1387.6 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **8073.0 SY**

STA 71+97.56 TO STA 75+84.13
AREA = 386.57 LF X 56.0' / 9 SF/CY = 2405.3 SY
ADDITIONAL AREA AT CURBS
STA 75+22.12 TO STA 75+53.12 LT =
31 LF X 2' / 9 SF/SY = 6.9 SY
STA 75+81.09 TO STA 76+05.09 RT =
24 LF X 2' / 9 SF/SY = 5.3 SY
2417.5 SY

EDGE COURSE = 386.57 LF X 2' = 773.14 LF
DEDUCT AT CURBS (31' + 24') = - 55.00 LF
717.14 LF

ITEM 446 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64=22
2417.5 SY X 1.25" / 36"/YD = **84.0 CY**

ITEM 446 ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
2417.5 SY X 1.75" / 36"/YD = **117.5 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
2417.5 SY X 8" / 36"/YD = 537.3 CY
EDGE COURSE =
718.14 LF X 0.67' X 0.5' AVG / 27 CF/CY = 8.9 CY
546.2 CY

ITEM 408 BITUMINOUS PRIME COAT
2417.5 SY X 0.4 GAL/SY= **967.0 GAL**

ITEM 304 AGGREGATE BASE
2417.5 SY X 6" / 36"/YD = 402.9 CY
EDGE COURSE 718.14 LF X 0.5' X 1.17' / 27 CF/CY = 15.6 CY
AT CURBS (31' + 24') X 0.5' X 1.5' / 27 CF/CY = 1.6 CY
420.1 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AND SHOULDERS = 2417.5 SY
ADD CURB (31' + 24') X 0.5' / 9 SF/SY = 3.1 SY
2420.6 SY

STA 75+84.13 TO STA 76+09.13
AREA = 25 LF X 63.0' / 9 SF/CY = 175.0 SY
EDGE COURSE = 25 LF X 2 SIDES = 50 LF

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=15")
175.0 SY

ITEM 304 AGGREGATE BASE
175.0 SY X 6" / 36"/YD = 29.2 CY
ADD EDGE COURSE 50 LF X 0.5' X 1' / 27 CF/CY = 0.9 CY
30.1 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **175.0 SY**

STA 78+61.13 TO STA 78+86.13
AREA = 25 LF X 63.0' / 9 SF/SY = 175.0 SY
EDGE COURSE = 25 LF X 2 SIDES = 50 LF

ITEM 611 REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=15")
175.0 SY

ITEM 304 AGGREGATE BASE
175.0 SY X 6" / 36"/YD = 29.2 CY
ADD EDGE COURSE 50 LF X 0.5' X 1' / 27 CF/CY = 0.9 CY
30.1 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **175.0 SY**

STA 78+86.13 TO STA 90+00
AREA STA 78+86.13 TO STA 88+00 =
913.87 LF X 56' / 9 SF/SY = 5686.3 SY
STA 88+00 TO STA 90+00
PAVEMENT 200 LF X 1/2 (36' + 24') / 9 SF/SY = 666.7 SY
SHOULDER LT
[150 LF X 10' + 50 LF X 1/2 (10' + 4')] / 9 SF/SY = 205.6 SY
SHOULDER RT
[180 LF X 10' + 100 LF X 1/2 (10' + 4)] / 9 SF/SY = 188.9 SY
ADD ADDITIONAL AREA STA 84+82.30 TO STA 85+
23.32 RT (COMPUTER) = 15.0 SY
ADD ADDITIONAL SHOULDERS AT CURB
STA 78+54.69 TO STA 78+79.69 LT =
25 LF X 2' / 9 SF/SY = 5.6 SY
STA 79+06.65 TO STA 79+37.65 RT =
31 LF X 2' / 9 SF/SY = 6.9 SY
6775.0 SY

EDGE COURSE 1113.87 LF X 2 SIDES = 2227.74 LF
DEDUCT CURBS (25' + 31') = - 56.00 LF
DEDUCT RAMP D EDGE LINE = - 76.17 LF
2095.57 LF

ITEM 446 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
6775.0 SY X 1.25" / 36"/YD = **235.2 CY**

ITEM 446 ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
6775.0 SY X 1.75" / 36"/YD = **329.3 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
6775.0 SY X 8" / 36"/YD = 1505.6 CY
EDGE COURSE
2095.57 LF X 0.67' X 0.5' AVG / 27 CF/CY = 26.0 CY
1531.6 CY

ITEM 408 BITUMINOUS PRIME COAT
6775.0 SY X 0.4 GAL/SY = **2710.0 GAL**

ITEM 304 AGGREGATE BASE
6775.0 SY X 6" / 36"/YD = 1129.2 CY
EDGE COURSE 2095.57 LF X 0.5' X 1.17' / 27 CF/CY = 45.4 CY
UNDER CURBS (25' + 31') X 0.5' X 1.5' / 27 CF/CY = 1.6 CY
1176.2 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **6775.0 SY**

SERVICE ROAD 1

STA 10+34.52 TO STA 13+00
PAVEMENT AREA = 265.48 LF X 24' / 9 SF/SY = 707.9 SY
SE RETURN (COMPUTER) = 46.1 SY
SW RETURN (COMPUTER) = 14.3 SY
768.3 SY

SHOULDER AREA
SE RETURN TO STA 10+86.07 (COMPUTER) = 24.8 SY
SW RETURN TO STA 10+86.07 (COMPUTER) = 27.0 SY
STA 10+86.07 TO STA 13+00 =
213.93 LF X 2 SIDES X 4' / 9 SF/SY = 190.2 SY
DEDUCT AREA OF DRIVE 8-DW IN SHOULDER (COMPUTER) = -32.2 SY
209.8 SY

EDGE COURSE
SE RETURN = 55.09 LF
SW RETURN = 37.64 LF
STA 10+64.21 TO STA 10+86.07 RT = 21.86 LF
STA 10+86.07 TO STA 13+00 = 213.93 LF X 2 SIDES = 427.86 LF
542.45 LF

ITEM 448 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
768.3 SY X 1.25" / 36"/YD = **26.7 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
768.3 SY X 3" / 36"/YD = 64.0 CY
EDGE COURSE 542.45 LF X 0.25' X 0.25' / 27 CF/SY = 1.3 CY
65.3 CY

ITEM 408 BITUMINOUS PRIME COAT
768.3 SY X 0.4 GAL/SY = **307.3 GAL**

ITEM 304 AGGREGATE BASE
768.3 SY X 6" / 36"/YD = 128.1 CY
EDGE COURSE 542.45 LF X .5' X .75' / 27 CF/CY = 7.5 CY
SHOULDER 209.8 SY X 8" / 36"/YD = 46.6 CY
DEDUCT AREA INCLUDED IN EDGE COURSE
542.45 LF X 0.3142 SF / 27 CF/CY = - 6.3 CY
175.9 CY

ITEM 203 SUBGRADE COMPACTION
PAVEMENT AREA = **768.3 SY**

SERVICE ROAD 2

STA 8+00 TO STA 9+58.83
PAVEMENT AREA = 158.83 LF X 24' / 9 SF/SY = 423.5 SY
NW RETURN (COMPUTER) = 20.3 SY
NE RETURN (COMPUTER) = 34.9 SY
478.7 SY

SHOULDER AREA
STA 8+00 TO STA 9+13.59 =
113.59 LF X 2 SIDES X 4' / 9 SF/SY = 101.0 SY
NW RETURN FROM STA 9+13.59 (COMPUTER) = 23.8 SY
NE RETURN (COMPUTER) = 22.8 SY
147.6 SY

EDGE COURSE
NW RETURN = 42.27 LF
STA 8+00 TO STA 9+23.78 RT = 123.78 LF
STA 8+00 TO STA 9+13.59 LT = 113.59 LF
NE RETURN = 50.47 LF
330.11 LF

ITEM 448 ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
478.7 SY X 1.25" / 36"/YD = **16.6 CY**

ITEM 301 BITUMINOUS AGGREGATE BASE, PG64-22
478.7 SY X 3" / 36"/YD = 39.9 CY
EDGE COURSE 330.11 LF X 0.25' X 0.25' / 27 CF/CY = 0.8 CY
40.7 CY

ITEM 408 BITUMINOUS PRIME COAT
478.7 SY X 0.4 GAL/SY = **191.5 GAL**

ITEM 304 AGGREGATE BASE
478.7 SY X 6" / 36"/YD = 79.8 CY
EDGE COURSE 330.11 LF X .5' X .75' / 27 CF/CY = 4.6 CY
SHOULDER 147.6 SY X 8" / 36"/YD = 32.8 CY
DEDUCT AREA INCLUDED IN EDGE COURSE
330.11 LF X 0.3142 SF / 27 CF/CY = - 3.8 CY
113.4 CY

CALCULATED M.T.
CHECKED W.G.Y.

PAVEMENT CALCULATIONS

FAY-35-2.57

Q:\3590\3590SS03

SERVICE ROAD 2, CON'T.

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **478.7 SY**

SERVICE ROAD 3

STA 16+75 TO STA 29+88

PAVEMENT AREA = 1313 LF X 24' / 9 SF/SY = 3501.3 SY
NW RETURN (COMPUTER) = 63.4 SY
NE RETURN (COMPUTER) = 63.4 SY
3628.1 SY

SHOULDER AREA
STA 16+75.00 TO STA 29+38.00
1263 LF X 2 SIDES X 4' / 9 SF/SY = 1122.7 SY
NW RETURN (COMPUTER) = 29.7 SY
NE RETURN (COMPUTER) = 29.7 SY
1182.1 SY

EDGE COURSE
STA 16+75 TO STA 29+38 = 1263 LF X 2 SIDES = 2526.00 LF
NW RETURN (COMPUTER) = 78.19 LF
NE RETURN (COMPUTER) = 78.19 LF
2682.38 LF

ITEM 448, ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
3628.1 SY X 1.25" / 36"/YD = **126.0 CY**

ITEM 301, BITUMINOUS AGGREGATE BASE, PG64-22
3628.1 SY X 3" / 36"/YD = 302.3 CY
EDGE COURSE 2682.38 LF X 0.25' X 0.25' / 9 SF/SY = 18.6 CY
320.9 CY

ITEM 408, BITUMINOUS PRIME COAT
3628.1 SY X 0.4 GAL/SY = **1451.2 GAL**

ITEM 304, AGGREGATE BASE
3628.1 SY X 6" / 36"/YD = 604.7 CY
EDGE COURSE 2682.38 LF X 0.5' X 0.75' / 9 SF/SY = 111.8 CY
SHOULDER 1182.1 SY X 8" / 36"/YD = 262.7 CY
DEDUCT AREA IN EDGE COURSE
2682.38 LF X 0.3142 SF / 9 SF/SY = - 93.6 CY
885.6 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT = **3628.1 SY**

RAMP C

STA 227+48.46 TO STA 230+59.33

AREA STA 227+48.46 TO STA 228+48.46 (COMPUTER) = 301.8 SY
STA 228+48.46 TO STA 230+10.04 (COMPUTER) = 451.9 SY
STA 230+10.04 TO STA 230+59.33 (COMPUTER) = 263.7 SY
1017.4 SY

EDGE COURSE
STA 227+48.46 TO STA 228+48.46
102.65 LF + 99.24 LF = 201.89 LF
STA 228+48.46 TO STA 230+10.04
164.80 LF + 160.57 LF = 325.37 LF
STA 230+10.04 TO STA 230+59.33
22.93 LF + 18.31 LF + 48.98 LF = 90.22 LF
617.48 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
1017.4 SY X 1.25" / 36"/YD = 35.3 CY
1017.4 SY X 1.75" / 36"/YD = 49.5 CY
1017.4 SY X 9" / 36"/YD = 254.4 CY
EDGE COURSE 617.48 LF X 0.03 CY/LF = 18.5 CY
357.7 CY

ITEM 408, BITUMINOUS PRIME COAT
[1017.4 SY - (617.48 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **356.7 GAL**

ITEM 304, AGGREGATE BASE
1017.4 SY X 10" / 36"/YD = 282.6 CY
EDGE COURSE 617.48 LF X 0.0375 CY/LF = 23.2 CY
305.8 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **1017.4 SY**

STA 230+59.33 TO STA 237+19.33

AREA (COMPUTER) = 2639.7 SY
EDGE COURSE = 660.00' + 659.72' = 1319.72 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
2639.7 SY X 1.25" / 36"/YD = 91.7 CY
2639.7 SY X 1.75" / 36"/YD = 128.3 CY
2639.7 SY X 9" / 36"/YD = 659.9 CY
EDGE COURSE 1319.72 LF X 0.03 CY/LF = 39.6 CY
919.5 CY

ITEM 408, BITUMINOUS PRIME COAT
[2639.7 SY -(1319.72 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **948.5 GAL**

ITEM 304 AGGREGATE BASE
2639.7 SY X 10" / 36"/YD = 733.3 CY
EDGE COURSE 1319.72 LF X 0.0375 CY/LF = 49.5 CY
782.8 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **2639.7 SY**

STA 237+19.33 TO STA 241+20.14

AREA 400.81 LF X 25' / 9 SF/SY = 1113.4 SY
EDGE COURSE 400.81 LF X 2 SIDES = 801.62 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
1113.4 SY X 1.25" / 36"/YD = 38.7 CY
1113.4 SY X 1.75" / 36"/YD = 54.1 CY
1113.4 SY X 9" / 36"/YD = 278.4 CY
EDGE COURSE 801.62 LF X 0.03 CY/LF = 24.0 CY
395.2 CY

ITEM 408, BITUMINOUS PRIME COAT
[1113.4 SY - (801.62' X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **380.2 GAL**

ITEM 304, AGGREGATE BASE
1113.4 SY X 10" / 36"/YD = 309.3 CY
EDGE COURSE 801.62 LF X 0.0375 CY/LF = 30.1 CY
339.4 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **1113.4 SY**

RAMP C ACCELERATION LANE STA 241+20.14 TO STA 256+07.25

AREA (COMPUTER) = 2593.1 SY
EDGE COURSE = 1513.91 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
2593.1 SY X 1.25" / 36"/YD = 90.0 CY
2593.1 SY X 1.75" / 36"/YD = 126.1 CY
2593.1 SY X 9" / 36"/YD = 648.3 CY
EDGE COURSE 1513.91 LF X 0.03 CY/LF = 45.4 CY
909.8 CY

ITEM 307, NON-STABILIZED DRAINAGE BASE, TYPE IA = **2593.1 SY**

ITEM 408, BITUMINOUS PRIME COAT
[2593.1 SY - (1513.91 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **914.1 GAL**

ITEM 304, AGGREGATE BASE
2593.1 SY X 6" / 36"/YD = 432.2 CY
EDGE COURSE 1513.91 LF X 0.0231 CY/LF = 35.0 CY
467.2 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **2593.1 SY**

RAMP C, STA 256+07.25 TO STA 258+00

AREA = ½(192.75')(12'+10') = 2120.25 SF ÷ 9 = 235.58 SY

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
(2120.25 SY)(1.25"/12)+27 = 8.18 CY
(2120.25 SF) (1.75"/12)+27 = 11.45 CY
(2120.25 SF) (9"/12)+27 = 58.90 CY
ADD OUTSIDE STEP:
(5"/12)(9"/12)(192.75 LF)+27 = 2.23 CY
80.76 CY

ITEM 307, 4" NON-STABILIZED DRAINAGE BASE, TYPE IA
(2120.25 SF)÷9 = **235.58 SY**

ITEM 304, 6" AGGREGATE BASE
(2120.25 SF)(6"/12)÷27 = 39.26 CY
ADD OUTSIDE STEP:
(17"/12)(6"/12)(192.75')÷27 = 5.07 CY
44.33 CY

ITEM 203, SUBGRADE COMPACTION
½(192.75')(12'+10')÷9 = **235.58 SY**

RAMP CC

STA 229+13.33 TO STA 230+10.32

AREA (COMPUTER) = 279.0 SY
EDGE COURSE = 186.63 LF

ITEM 446, ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22
279.0 SY X 1.25" / 36"/YD = **9.7 CY**

ITEM 446, ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
279.0 SY X 1.75" / 36"/YD = **13.6 CY**

ITEM 301, BITUMINOUS AGGREGATE BASE, PG64-22
279.0 SY X 9" / 36"/YD = 69.8 CY
EDGE COURSE 186.63 LF X 0.03 CY/LF = 5.6 CY
75.4 CY

ITEM 408, BITUMINOUS PRIME COAT
[279.0 SY - (186.63 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **96.4 GAL**

ITEM 304, AGGREGATE BASE
279.0 SY X 10" / 36"/YD = 77.5 CY
EDGE COURSE 186.63 LF X 0.0375 CY/LF = 7.0 CY
84.5 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **279.0 SY**

ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE
(279 SY)(0.010 GAL/SY) = **280.0 GAL.**

RAMP D

STA 237+22.03 TO PC STA 238+00

AREA (COMPUTER) = 340.6 SY
EDGE COURSE (63.30' + 10.38' + 56.15' + 34.00') = 163.83 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
340.6 SY X 1.25" / 36"/YD = 11.8 CY
340.6 SY X 1.75" / 36"/YD = 16.6 CY
340.6 SY X 9" / 36"/YD = 85.2 CY
EDGE COURSE 163.83 LF X 0.03 CY/LF = 4.9 CY
118.5 CY

ITEM 408, BITUMINOUS PRIME COAT
[340.6 SY - (163.83 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **122.9 GAL**

ITEM 304, AGGREGATE BASE
340.6 SY X 10" / 36"/YD = 94.6 CY
EDGE COURSE 163.83 LF X 0.0375 CY/LF = 6.2 CY
100.8 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **340.6 SY**

STA 238+00 TO STA 248+47.74

AREA (COMPUTER) = 2929.5 SY
EDGE COURSE 943.78' + 49.84' + 49.86' + 101.35' + 960.32' = 2105.15 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
2929.5 SY X 1.25" / 36"/YD = 101.7 CY
2929.5 SY X 1.75" / 36"/YD = 142.4 CY
2929.5 SY X 9" / 36/YD = 732.4 CY
EDGE COURSE:
2105.15 LF X 0.03 CY/LF = 63.2 CY
1039.7 CY

ITEM 408, BITUMINOUS PRIME COAT
[2929.5 SY - (2105.15 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **1000.6 GAL**

ITEM 304, AGGREGATE BASE
2929.5 SY X 10" / 36"/YD = 813.8 CY
EDGE COURSE 2105.15 LF X 0.0375 CY/LF = 78.9 CY
892.7 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **2929.5 SY**

CALCULATED M.T. CHECKED W.G.Y.
REVISED 12/99 BY FYC

PAVEMENT CALCULATIONS

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\fay47gc.dgn 01-FEB-2000 2:41PM fcheek
I:\PR\PROJECT\FAY35\FAY-35-2.57\FAY47GC

RAMP D DECELERATION LANE: STA 248+47.74 TO STA 256+38.05

AREA (COMPUTER) = 1108.6 SY
EDGE COURSE 450.36' + 233.54' + 98.53' = 782.43 LF

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
1108.6 SY X 1.25" / 36"/YD = 38.5 CY
1108.6 SY X 1.75" / 36"/YD = 53.9 CY
1108.6 SY X 9" / 36"/YD = 277.2 CY
EDGE COURSE 782.43 LF X 0.03 CY/LF = 23.5 CY
393.1 CY

ITEM 307, NON-STABILIZED DRAINAGE BASE, TYPE IA = **1108.6 SY**

ITEM 408, BITUMINOUS PRIME COAT
[1108.6 SY - (782.43 LF X 1.83' / 9 SF/SY)] X 0.4 GAL/SY = **379.8 GAL**

ITEM 304, AGGREGATE BASE
1108.6 SY X 6" / 36"/YD = 184.8 CY
EDGE COURSE 782.43 LF X 0.0231 CY/LF = 18.1 CY
202.9 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **1108.6 SY**

SR 729

STA 152+75 TO STA 165+53.08

PAVEMENT AREA STA 152+75 TO STA 153+85
110 LF X 1/2 (20' + 22') / 9 SF/SY = 256.7 SY
STA 153+85 TO STA 165+53.08
1168.08 LF X 22' / 9 SF/SY = 2855.3 SY
ADDED AREA AT CURBS
STA.165+34.27 TO STA. 165+58.27 LT
24 LF X 6' / 9 SF/SY = 16.0 SY
STA 165+23.91 TO STA 165+47.91 RT
24 LF X 6' / 9 SF/SY = 16.0 SY
3144.0 SY

EDGE COURSE
STA 152+75 TO STA 165+34.27 LT = 1259.27 LF
STA 152+75 TO STA 165+23.91 RT = 1248.91 LF
2508.18 LF

SHOULDER AREA
STA 152+75 TO STA 153+00
25 LF X 2 SIDES X 1/2(3' + 4') / 9 SF/SY = 19.4 SY
STA 153+00 TO STA 165+34.27 LT
1234.27' X 4' / 9 SF/SY = 548.6 SY
STA 153+00 TO STA 165+23.91 RT
1223.91' X 4' / 9 SF/SY = 544.0 SY
DEDUCT AREA INCLUDED IN DRIVE 10-DW (COMPUTER) = -29.2 SY
DEDUCT AREA INCLUDED IN DRIVE 12-DW (COMPUTER) = -22.5 SY
1060.3 SY

ITEM 446, ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
3144.0 SY X 1.25" / 36"/YD = **109.2 CY**

ITEM 446, ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
3144.0 SY X 1.75" / 36"/YD = **152.8 CY**

ITEM 301, BITUMINOUS AGGREGATE BASE, PG64-22
3144.0 SY X 4" / 36"/YD = 349.3 CY
ADD EDGE COURSE
2508.18 LF X .33' X .33' / 27 CF/CY = 10.1 CY
359.4 CY

ITEM 408, BITUMINOUS PRIME COAT
3144.0 SY X 0.4 GAL/SY = **1257.6 GAL**

ITEM 304, AGGREGATE BASE
PAVEMENT 3144.0 SY X 6" / 36"/YD = 524.0 CY
EDGE COURSE 2508.18 LF X 0.5' X 0.83' / 27 CF/CY = 38.6 CY
SHOULDER 1060.3 SY X 8" / 36"/YD = 235.6 CY
DEDUCT AREA INCLUDED IN EDGE COURSE
2508.18 LF X 0.0073 CY/LF = - 18.3 CY
779.9 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **3144.0 SY**

ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE
(0.010 GAL/SY)(3144 SY) = **31.4 GAL.**

STA 165+53.08 TO STA 165+73.08

AREA = 20 LF X 37' / 9 SF/SY = 82.2 SY
EDGE COURSE = 20 LF X 2 SIDES = 40 LF

ITEM 611, REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=13")
AREA = **82.2 SY**

ITEM 304, AGGREGATE BASE
82.2 SY X 6" / 36"/YD = 13.7 CY
EDGE COURSE 40 LF X 0.5' X 1.0' / 27 CF/CY = 0.7 CY
14.4 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT = **82.2 CY**

STA 168+25.18 TO STA 168+45.18

AREA = 20 LF X 37' / 9 SF/SY = 82.2 SY
EDGE COURSE = 20 LF X 2 SIDES = 40 LF

ITEM 611, REINFORCED CONCRETE APPROACH SLAB, AS PER PLAN, (T=13")
AREA = **82.2 CY**

ITEM 304, AGGREGATE BASE
82.2 SY X 6" / 36"/YD = 13.7 CY
EDGE COURSE: 40 LF X 0.5' X 1' / 27 CF/CY = 0.7 CY
14.4 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT = **82.2 CY**

STA 168+45.18 TO STA 181+44.00

PAVEMENT AREA: STA 168+45.18 TO STA 181+00:
1254.82 LF X 22' / 9 SF/SY = 3067.3 SY
STA 181+00 TO STA 181+44:
44 LF X 1/2(22' + 24.62') / 9 SF/SY = 114.0 SY
ADDED AREA AT CURBS
STA 168+40.00 TO STA 168+64.00 RT:
24' X 6' / 9 SF/SY = 16.0 SY
STA 168+50.37 TO STA 168+74.37 LT:
24' X 6' / 9 SF/SY = 16.0 SY
3213.3 SY

EDGE COURSE
STA 168+64.00 TO STA 181+44.00 RT = 1280.00 LF
STA 168+74.37 TO STA 181+44.00 LT = 1269.63 LF
2549.63 LF

SHOULDER AREA
STA 168+64.00 TO STA 181+44.00 RT:
1280.00 LF X 4' / 9 SF/SY = 568.9 SY
STA 168+74.37 TO STA 181+44.00 LT:
1269.63 LF X 4' / 9 SF/SY = 564.3 SY
1133.2 SY

ITEM 446, ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22
3213 SY X 1.25" / 36"/YD = **111.6 CY**

ITEM 446, ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
3213 SY X 1.75" / 36"/YD = **156.2 CY**

ITEM 301, BITUMINOUS AGGREGATE BASE, PG64-22
3213.3 SY X 4" / 36"/YD = 357.0 CY
EDGE COURSE:
2549.63 LF X 0.33' X 0.33' / 27 CF/CY = 10.3 CY
367.3 CY

ITEM 408, BITUMINOUS PRIME COAT
3213.3 SY X 0.4 GAL/SY = **1285.3 GAL**

ITEM 304, AGGREGATE BASE
PAVEMENT: 3213.3 SY X 6" / 36"/YD = 535.6 CY
EDGE COURSE:
2549.63 LF X 0.5' X 0.83' / 27 CF/CY = 39.2 CY
SHOULDER: 1133.2 SY X 8" / 36"/YD = 251.8 CY
DEDUCT AREA INCLUDED IN EDGE COURSE:
2549.63 LF X 0.0073 CY/LF = -18.6 CY
808.0 CY

ITEM 203, SUBGRADE COMPACTION
PAVEMENT AREA = **3213.3 SY**

ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE
(0.010 GAL/SY)(3213.3 SY) = **32.13 GAL.**

U-TURN MEDIAN CROSSOVER

AREA = (18' + 18' + 16') X 36' - π X 18^2 = 854.1 SF = 94.9 SY

EDGE COURSE = 113.1 LF

LOCATION: STA. 280+28
STA. 382+53

ITEM 880, ASPHALT CONCRETE (7 YEAR WARRANTY)
94.9 SY X 12" / 36"/YD = 31.6 CY

EDGE COURSE:
113.1 LF X 0.75' X 0.75' / 27 CF/CY = 2.4 CY
34.0 CY

ITEM 408, BITUMINOUS PRIME COAT
94.9 SY X 0.4 GAL/SY = **38.0 GAL**

ITEM 304, AGGREGATE BASE
94.9 SY X 10" / 36"/YD = 26.4 CY
EDGE COURSE:
113.1 LF X 0.83' X 1.25' / 27 CF/CY = 4.3 CY
30.7 CY

ITEM 203, SUBGRADE COMPACTION
AREA = **94.9 GAL**

CALCULATED M.T.
CHECKED W.G.Y.
REVISED 12/99
BY FYC

PAVEMENT CALCULATIONS

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\fay48gc.dgn 01-FEB-2000 2:37PM fcheek
I:\pr\project\fay35\fay-35-2.57\fay48gc.dgn

ITEM 448 - 1¼" ASPHALT CONCRETE SURFACE COURSE, TYPE I, PG64-22

STA. 24+50 TO STA. 25+63 = 113.0 L.F.
(1.25"/12)[½(113')(15.01'+20')]÷27 = 7.63 C.Y.

STA. 25+63 TO STA. 29+24 = 361 L.F.
([(361')(20.0')(1.25/12)]÷27 = 27.86 C.Y.

STA. 29+24 TO STA. 30+44 = 120 L.F.
(1.25"/12)[½(120')(14.67'+20')]÷27 = 8.03 C.Y.

TOTAL = 43.52 C.Y.
(Use 44 C.Y.)

ITEM 448 - 1¾" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22

STA. 24+50 TO STA. 25+63 = 113.0 L.F.
(1.75"/12)[½(113')(15.01'+20')]÷27 = 10.68 C.Y.

STA. 25+63 TO STA. 29+24 = 361 L.F.
[(361')(20')(1.75"/12)]÷27 = 39.00 C.Y.

STA. 29+24 TO STA. 30+44 = 120 L.F.
(1.75"/12)[½(120')(14.67'+20')]÷27 = 11.23 C.Y.

TOTAL = 61.67 C.Y.
(Use 62 C.Y.)

ITEM 411 - 8" STABILIZED CRUSHED AGGREGATE

STA. 25+06.56 TO STA. 25+63 = 56.44 L.F.
(8"/12)[½(56.44')(2'+4')]÷27 * 2 = 8.36 C.Y.

STA. 25+63 TO STA. 29+24 = 361 L.F.
[(2)(361')(4')(8"/12)]÷27 = 71.31 C.Y.

STA. 29+24 TO STA. 30+44 = 120 L.F.
(8"/12)[½(60.71')(2'+4')]÷27 * 2 = 8.99 C.Y.

TOTAL = 88.66 C.Y.
(Use 89 C.Y.)

ITEM 408 - BITUMINOUS PRIME COAT (0.40 GAL/SY)

STA. 24+50 TO STA. 25+63 = 113.0 L.F.
[½(113')(15.01'+20')÷9]*(0.40 Gal./S.Y.) = 87.91 GAL.

STA. 25+63 TO STA. 29+24 = 361 L.F.
[(361')(20')÷9]*(0.40 Gal./S.Y.) = 320.89 GAL.

STA. 29+24 TO STA. 30+44 = 120 L.F.
[½(120')(14.67'+20')÷9]*(0.40 Gal./S.Y.) = 92.45 GAL.

TOTAL = 501.28 GAL.
(Use 501 GAL.)

ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE

STA. 24+50 TO STA. 25+63 = 113.0 L.F.
[½(113')(15.01'+20')÷9]*(0.10 Gal./S.Y.) = 21.98 GAL.

STA. 25+63 TO STA. 29+24 = 361 L.F.
[(361')(20')÷9]*(0.10 Gal./S.Y.) = 80.22 GAL.

STA. 29+24 TO STA. 30+44 = 120 L.F.
[½(120')(14.67'+20')÷9]*(0.10 Gal./S.Y.) = 23.11 GAL.

TOTAL = 125.31 GAL.
(Use 125 C.Y.)

ITEM 304 - 8" AGGREGATE BASE

STA. 24+50 TO STA. 25+63 = 113 L.F.
(8"/12)[½(113')(16.01+21.00')]÷27 = 51.63 C.Y.

STA. 25+63 TO STA. 29+24 = 361 L.F.
(8"/12)[(361')(21.00')]÷27 = 187.19 C.Y.

STA. 29+24 TO STA. 30+44 = 120 L.F.
(8"/12)[½(120')(15.67+21.00')]÷27 = 54.33 C.Y.

TOTAL = 293.15 C.Y.
(Use 293 C.Y.)

ITEM 203 - SUBGRADE COMPACTION

STA. 24+50 TO STA. 25+63 = 113.0 L.F.
½(113.0')(15.01'+17.51')÷9 = 204.15 SQ.YD.

STA. 25+63 TO STA. 29+24 = 361 L.F.
(20.0')(361')÷9 = 802.20 SQ.YD.

STA. 29+24 TO STA. 30+44 = 120.0 L.F.
½(120.0')(14.67'+20.0')÷9 = 231.13 SQ.YD.

TOTAL = 1237.48 SQ.YD.
(Use 1238 SQ.YD.)

CALCULATED 11/99 CHECKED FYC

PAVEMENT CALCULATIONS - HARMONY ROAD

FAY-35-2.57

PAVEMENT

48A
522

I:\pr\project\fay35\fay-35-2.57\gcharmny.dgn 03-DEC-1999 9:16AM fcheek

CALCULATIONS FOR REMOVAL OF UNSUITABLE MATERIAL & REPLACEMENT

Topsoil removal and replacement with suitable embankment material.

U.S.-35
Average Width = 140' & Average depth = 1'

Sta. 135+75 to 145+00	=	925'
Sta. 172+50 to 231+00	=	5850'
Sta. 241+25 to 251+00	=	975'
Sta. 254+00 to 273+00	=	1850'
Sta. 276+00 to 287+25	=	1125'
Sta. 290+00 to 360+00	=	7000'
Sta. 378+00 to 389+00	=	1100'
Sta. 395+00 to 396+25	=	125'
Deduct for Bridge over I-71	=	266'
Deduct for Bridge over C.R. 85	=	155'
		18,529 Lin. Ft.
18,529' x 140' x 1' / 27	=	96,076 Cu. Yd.

Ramp 'C'
Average Width = 60' & Average Depth = 1'
Sta. 227+50 to 238+00 = 1,050 Cu. Yd.
1050' x 60' x 1' / 27 = 2,333 Cu. Yd.

Ramp 'D'
Average Width = 55' & Average Depth = 1'
Sta. 237+50 to 248+00 = 1,050'
1050' x 55' x 1' / 27 = 2,139 Cu. Yd.

U.S.-35	Ramp 'C'	Ramp 'D'	Total
96,076 Cu.Yd.	+ 2,333 Cu. Yd.	+ 2,139 Cu.Yd.	= 100,548 Cu. Yd.

The following items are carried to the general summary for topsoil removal and replacement:

Item 203 - Excavation, not including Embankment Construction	= 100,548 Cu. Yd.
Item 203 - Embankment	= 100,548 Cu. Yd.

The following areas require the removal of unsuitable material below the proposed pavement subgrade & replacing with suitable embankment. The width is based on shoulder to shoulder plus one foot beyond; 1 side = 43'; 2 sides = 86'.

U.S. 35

Sta. 145+00 to 155+00	=	1000' x 2 sides
Sta. 231+00 to 238+00	=	700' x 2 sides
Sta. 251+00 to 254+50	=	350' x 1 side (left)
Sta. 273+00 to 276+00	=	300' x 1 side (right)
Sta. 360+00 to 378+00	=	1800' x 2 sides
Sta. 389+00 to 395+00	=	600' x 2 sides

1 side	= 43' x 650' x 1.5' / 27	= 1,553 Cu. Yd.
2 sides	= 86' x 4100' x 2.0' / 27	= 26,119 Cu. Yd.
		27,672 Cu. Yd.

The following items are carried to the general summary for the removal of unsuitable subgrade material :

Item 203 - Excavation of Unsuitable Material	= 27,672 Cu. Yd.
Item 203 - Embankment	= 27,672 Cu. Yd.

Subgrade Stabilization
The following quantities to be used to stabilize the subgrade "As Directed by the Engineer":

Item	Description	Quantity
206	12" Lime-Soil Stabilized Subgrade-Dry or Slurry Method	25,000 Sq. Yd.
206	Lime-applied at the rate of 30 pounds per Sq. Yd.(25,000 Sq. Yds. X 30# / S.Y. / 2000# / Ton)	375 Tons
206	Water-applied at the rate of 8 gallons per Sq. Yd.(25,000 Sq. Yds. x 8 Gal./ S.Y. / 1000 / M. Gal.)	200 M. Gal.

Soft Subgrade and Embankment Foundation

The following quantities are to be used for the removal of soft subgrade and replacement of embankment foundation soils "As Directed by the Engineer".

Item	Description	Quantity
203	Excavation of Unsuitable Material	15,000 Cu. Yd.
203	Embankment	5,000 Cu. Yd.
203	Embankment Using Granular Material	10,000 Cu. Yd.

Item 605 - Deep Pipe Underdrains

The following quantity has been added to help promote drainage of wet soils. In order to promote drainage this item should be installed soon after rough grading. The underdrains should be 50 inches deep in soil cuts.

Item	Description	Quantity
605	6" Deep Pipe Underdrains	20,000 Lin. Ft.

Haul Roads and Soft Subgrade

Some areas may have soft subgrade that is unsuitable for use as a haul road. If such areas are used as a haul road, the Contractor shall be responsible for any failures and additional cost to the State.

CALCULATED | CHECKED

REMOVAL OF UNSUITABLE MATERIALS

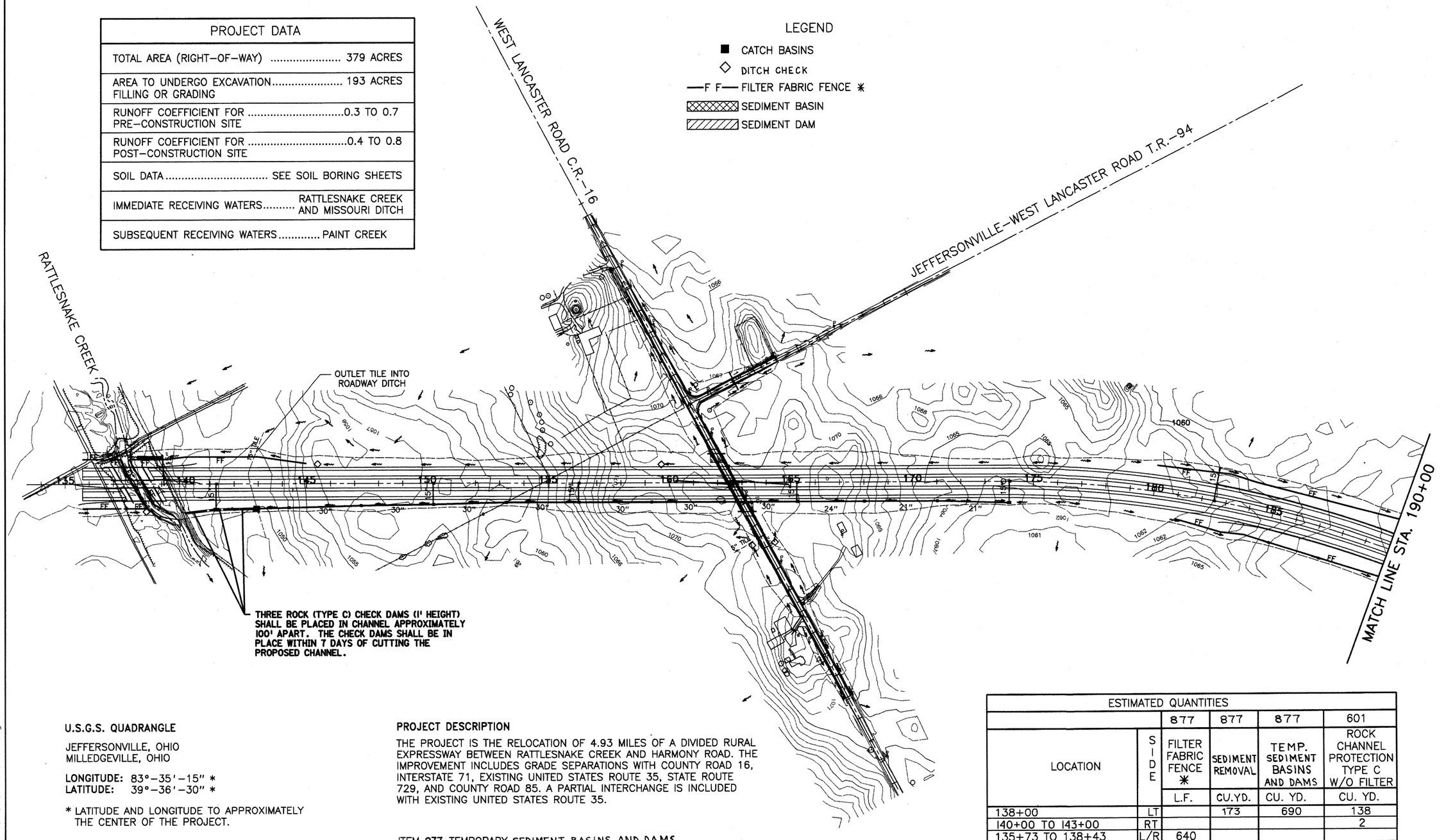
FAY-35-2.57

i:\users\dpoiling\fay\035\0257.000\48a.dgn 06-OCT-2000 2:34PM dpoiling

PROJECT DATA	
TOTAL AREA (RIGHT–OF–WAY)	379 ACRES
AREA TO UNDERGO EXCAVATION FILLING OR GRADING	193 ACRES
RUNOFF COEFFICIENT FOR PRE–CONSTRUCTION SITE	0.3 TO 0.7
RUNOFF COEFFICIENT FOR POST–CONSTRUCTION SITE	0.4 TO 0.8
SOIL DATA	SEE SOIL BORING SHEETS
IMMEDIATE RECEIVING WATERS	RATTLESNAKE CREEK AND MISSOURI DITCH
SUBSEQUENT RECEIVING WATERS	PAINT CREEK

LEGEND

- ■ CATCH BASINS
- ◇ DITCH CHECK
- —F F— FILTER FABRIC FENCE ✳
- SEDIMENT BASIN
- SEDIMENT DAM



THREE ROCK (TYPE C) CHECK DAMS (1' HEIGHT) SHALL BE PLACED IN CHANNEL APPROXIMATELY 100' APART. THE CHECK DAMS SHALL BE IN PLACE WITHIN 7 DAYS OF CUTTING THE PROPOSED CHANNEL.

U.S.G.S. QUADRANGLE

JEFFERSONVILLE, OHIO
MILLEDGEVILLE, OHIO

LONGITUDE: 83°–35'–15" *
LATITUDE: 39°–36'–30" *

* LATITUDE AND LONGITUDE TO APPROXIMATELY THE CENTER OF THE PROJECT.

PROJECT DESCRIPTION

THE PROJECT IS THE RELOCATION OF 4.93 MILES OF A DIVIDED RURAL EXPRESSWAY BETWEEN RATTLESNAKE CREEK AND HARMONY ROAD. THE IMPROVEMENT INCLUDES GRADE SEPARATIONS WITH COUNTY ROAD 16, INTERSTATE 71, EXISTING UNITED STATES ROUTE 35, STATE ROUTE 729, AND COUNTY ROAD 85. A PARTIAL INTERCHANGE IS INCLUDED WITH EXISTING UNITED STATES ROUTE 35.

ITEM 877 TEMPORARY SEDIMENT BASINS AND DAMS

THE SEDIMENT BASIN QUANTITIES LISTED ON THIS STORM WATER POLLUTION PREVENTION PLAN ARE THE STORAGE VOLUMES REQUIRED FOR THE SEDIMENT BASIN. THE PAY QUANTITY FOR EACH BASIN SHALL BE DETERMINED AS THE ACTUAL AMOUNT OF EXCAVATION OR EMBANKMENT REQUIRED TO PROVIDE THAT STORAGE VOLUME

ESTIMATED QUANTITIES

LOCATION	SIDE	877 FILTER FABRIC FENCE ✳ L.F.	877 SEDIMENT REMOVAL CU.YD.	877 TEMP. SEDIMENT BASINS AND DAMS CU. YD.	601 ROCK CHANNEL PROTECTION TYPE C W/O FILTER CU. YD.
138+00	LT		173	690	138
140+00 TO 143+00	RT				2
135+73 TO 138+43	L/R	640			
137+66 TO 145+00	L/R	1090			
180+00 TO 190+00	LT	1025			
181+00 TO 190+00	RT	880			
AT CATCH BASINS		1220			
AT DITCH CHECKS		75			
TOTAL		4930	173	690	140

✳ TEMPORARY PERIMETER, DITCH CHECK, OR INLET PROTECTION FILTER FABRIC FENCE

HORIZONTAL SCALE IN FEET: 0, 100, 200, 400

CALCULATED | CHECKED

REVISED 10-99 BY FYC

STORM WATER POLLUTION PREVENTION PLAN
STA. 134+00 TO STA. 190+00

FAY–35–2.57

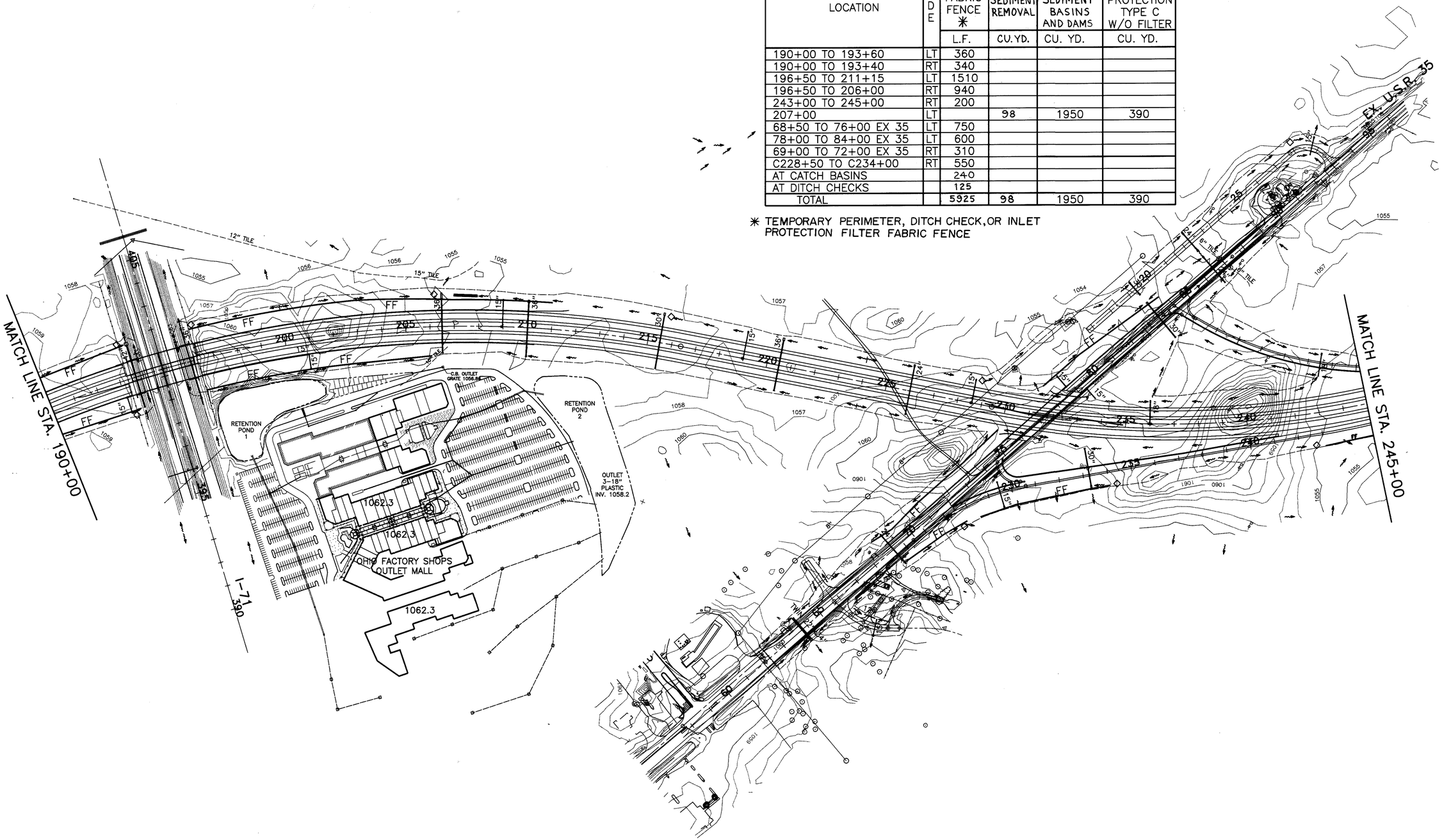
49
522

i:\users\dwise\fay\35\257&75\40lnewsht\new049.dgn 20-MAR-2001 3:36PM dwise

REVISED 1-31-01 DSW

ESTIMATED QUANTITIES					
		877	877	877	601
LOCATION	SIDE	FILTER FABRIC FENCE *	SEDIMENT REMOVAL	TEMP. SEDIMENT BASINS AND DAMS	ROCK CHANNEL PROTECTION TYPE C W/O FILTER
		L.F.	CU.YD.	CU. YD.	CU. YD.
190+00 TO 193+60	LT	360			
190+00 TO 193+40	RT	340			
196+50 TO 211+15	LT	1510			
196+50 TO 206+00	RT	940			
243+00 TO 245+00	RT	200			
207+00	LT		98	1950	390
68+50 TO 76+00 EX 35	LT	750			
78+00 TO 84+00 EX 35	LT	600			
69+00 TO 72+00 EX 35	RT	310			
C228+50 TO C234+00	RT	550			
AT CATCH BASINS		240			
AT DITCH CHECKS		125			
TOTAL		5925	98	1950	390

* TEMPORARY PERIMETER, DITCH CHECK, OR INLET PROTECTION FILTER FABRIC FENCE



REVISED 10-99 BY FYC
CALCULATED
CHECKED

HORIZONTAL SCALE IN FEET
0 100 200 400

STORM WATER POLLUTION PREVENTION PLAN
STA. 190+00 TO STA. 245+00

FAY-35-2.57

50
522

ESTIMATED QUANTITIES					
		877			
LOCATION	SIDE	FILTER FABRIC FENCE ✳			
		L.F.			
245+00 TO 249+50	RT	460			
259+50 TO 261+00	RT	160			
263+50 TO 265+00	RT	150			
270+00 TO 272+00	RT	200			
157+00 TO 160+50 SR729	LT	350			
158+50 TO 161+00 SR729	RT	250			
174+50 TO 176+50 SR729	RT	200			
AT CATCH BASINS		100			
AT DITCH CHECKS		138			
TOTAL		2008			

✳ TEMPORARY PERIMETER, DITCH CHECK, OR INLET PROTECTION FILTER FABRIC FENCE

EX. U.S.R. 35

MATCH LINE STA. 245+00

WETLAND

WETLAND SITE C

MISSOURI DITCH

MATCH LINE STA. 300+00

S.R. 729

0 100 200 400 HORIZONTAL SCALE IN FEET

REVISED 10-99 BY FYC

CALCULATED

CHECKED

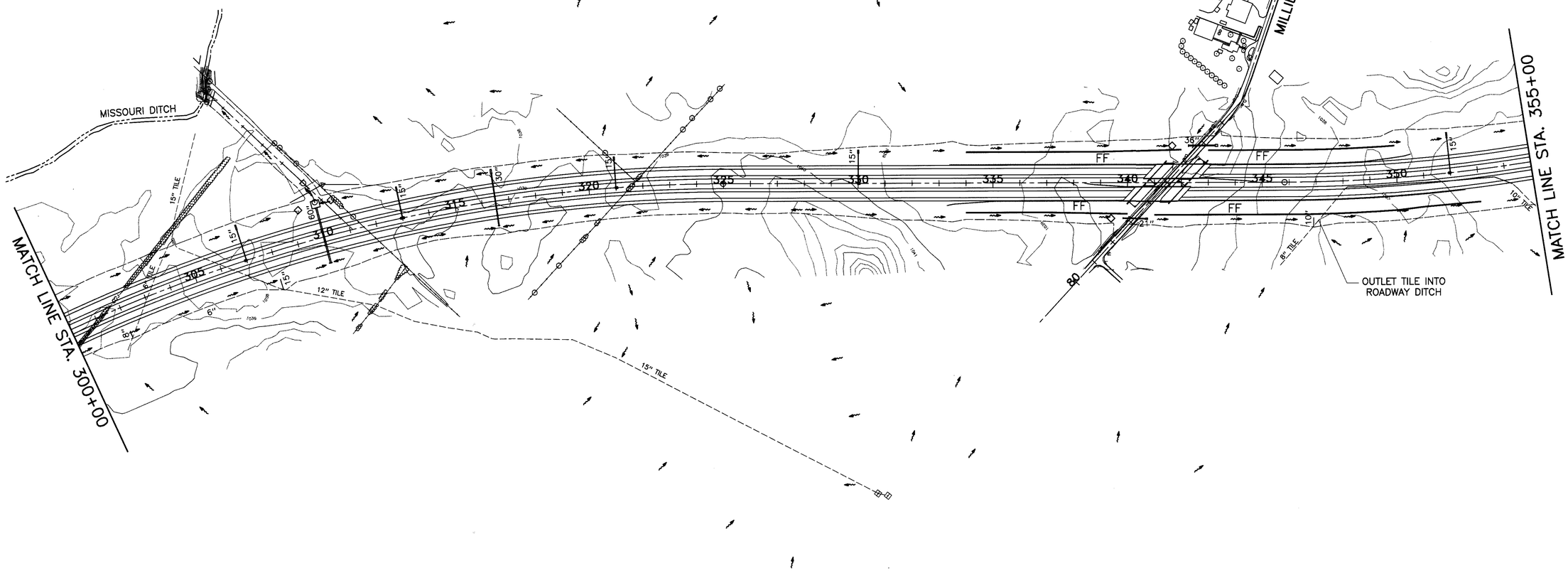
STORM WATER POLLUTION PREVENTION PLAN
STA. 245+00 TO STA. 300+00

FAY-35-2.57

51
522

ESTIMATED QUANTITIES					
		877			
LOCATION	SIDE	FILTER FABRIC FENCE *			
		L.F.			
334+00 TO 340+00	RT	600			
334+00 TO 342+00	LT	800			
341+00 TO 353+00	RT	1200			
343+00 TO 350+00	LT	700			
AT CATCH BASINS		180			
AT DITCH CHECKS		50			
TOTAL		3530			

* TEMPORARY PERIMETER, DITCH CHECK, OR INLET PROTECTION FILTER FABRIC FENCE



HORIZONTAL SCALE IN FEET: 0, 100, 200, 400

CALCULATED | CHECKED

REVISED 10-99 BY FYC

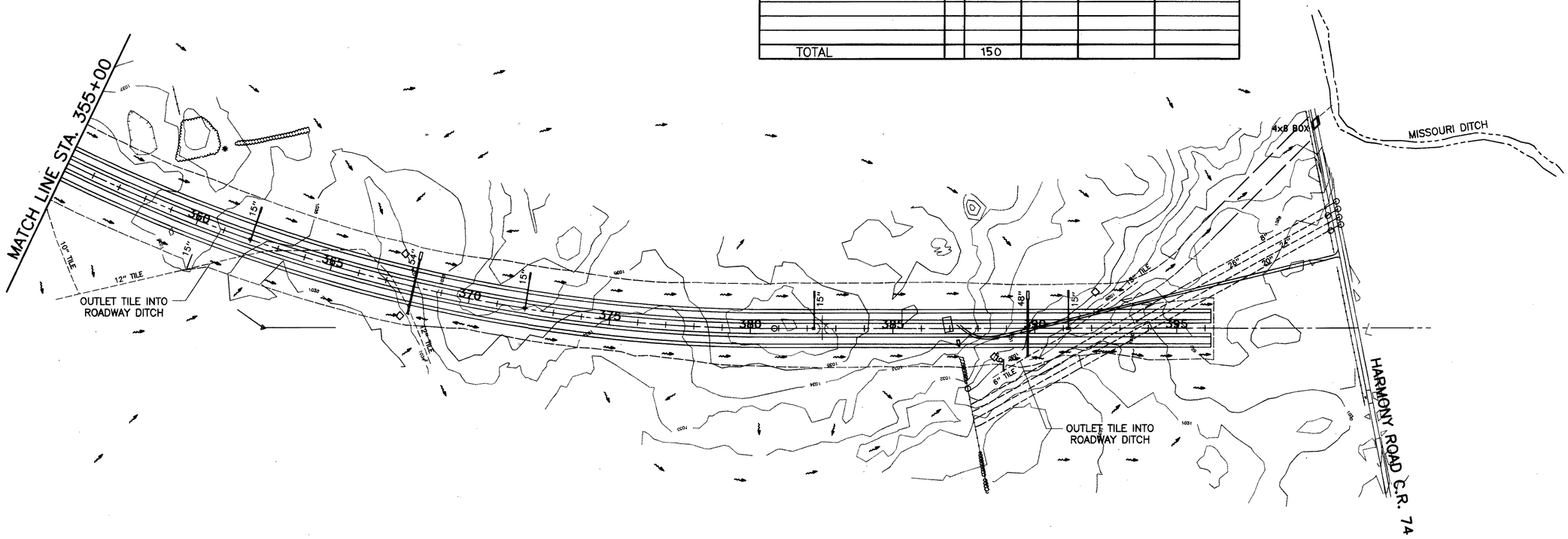
STORM WATER POLLUTION PREVENTION PLAN
STA. 300+00 TO STA. 355+00

FAY-35-2.57

52
522

ESTIMATED QUANTITIES					
		877			
LOCATION	SIDE	FILTER FABRIC FENCE *			
		L.F.			
AT CATCH BASINS		100			
AT DITCH CHECKS		50			
TOTAL		150			

* TEMPORARY PERIMETER, DITCH CHECK, OR INLET PROTECTION FILTER FABRIC FENCE



REVISED	10-99
BY	FYC
CALCULATED	
CHECKED	

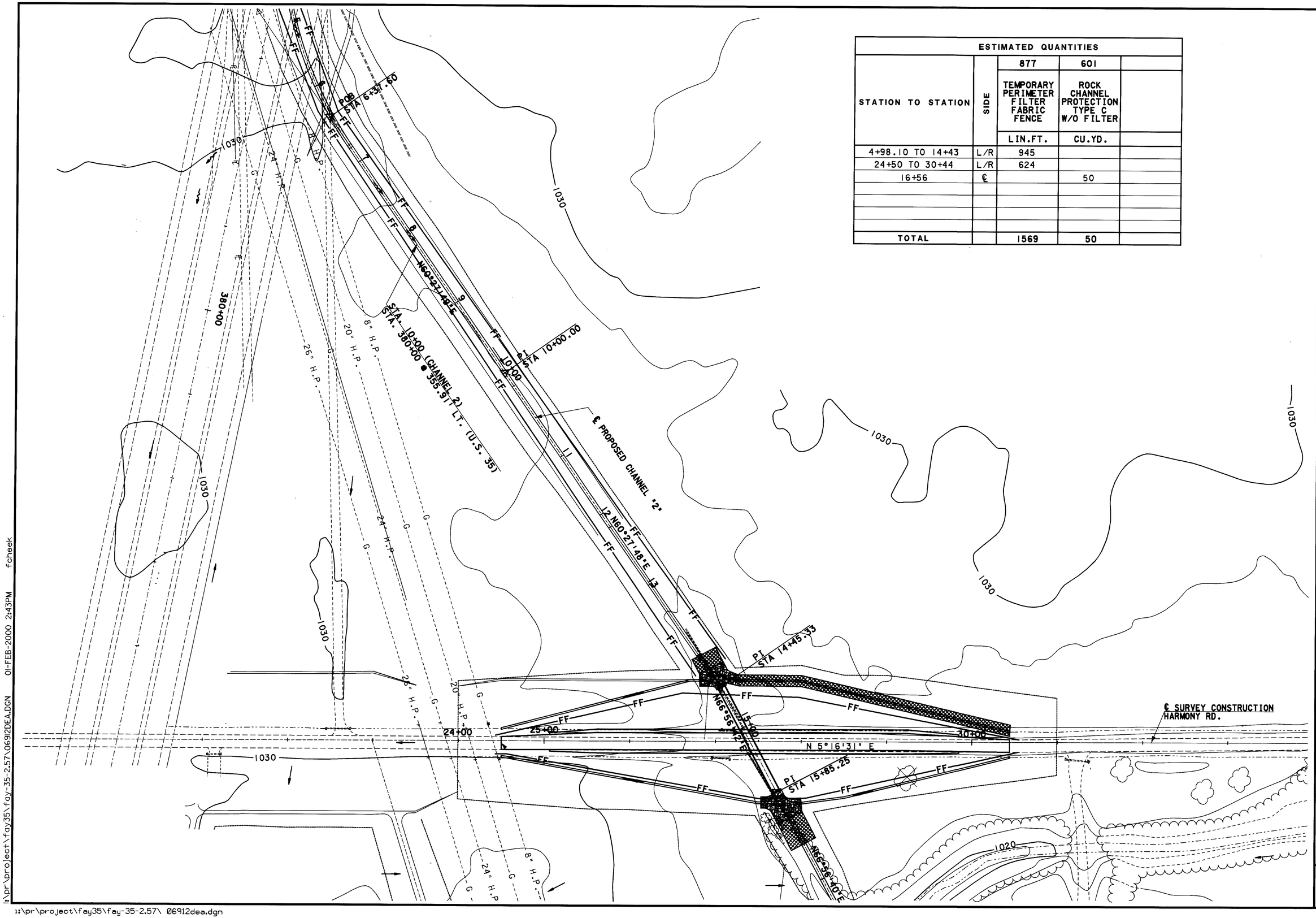
0 100 200 400 HORIZONTAL SCALE IN FEET

STORM WATER POLLUTION PREVENTION PLAN
STA. 355+00 TO STA. 397+00

FAY-35-2.57

53
522

ESTIMATED QUANTITIES				
		877	601	
STATION TO STATION	SIDE	TEMPORARY PERIMETER FILTER FABRIC FENCE	ROCK CHANNEL PROTECTION TYPE C W/O FILTER	
		LIN.FT.	CU.YD.	
4+98.10 TO 14+43	L/R	945		
24+50 TO 30+44	L/R	624		
16+56	℄		50	
TOTAL		1569	50	



STORM WATER POLLUTION PREVENTION PLAN
HARMONY ROAD

REVISED 1/00 BY FYC

HORIZONTAL SCALE IN FEET

FAY-35-2.57

53A
522

i:\pr\project\fay35\fay-35-2.57\06912DEA.DGN 01-FEB-2000 2:43PM fcheek

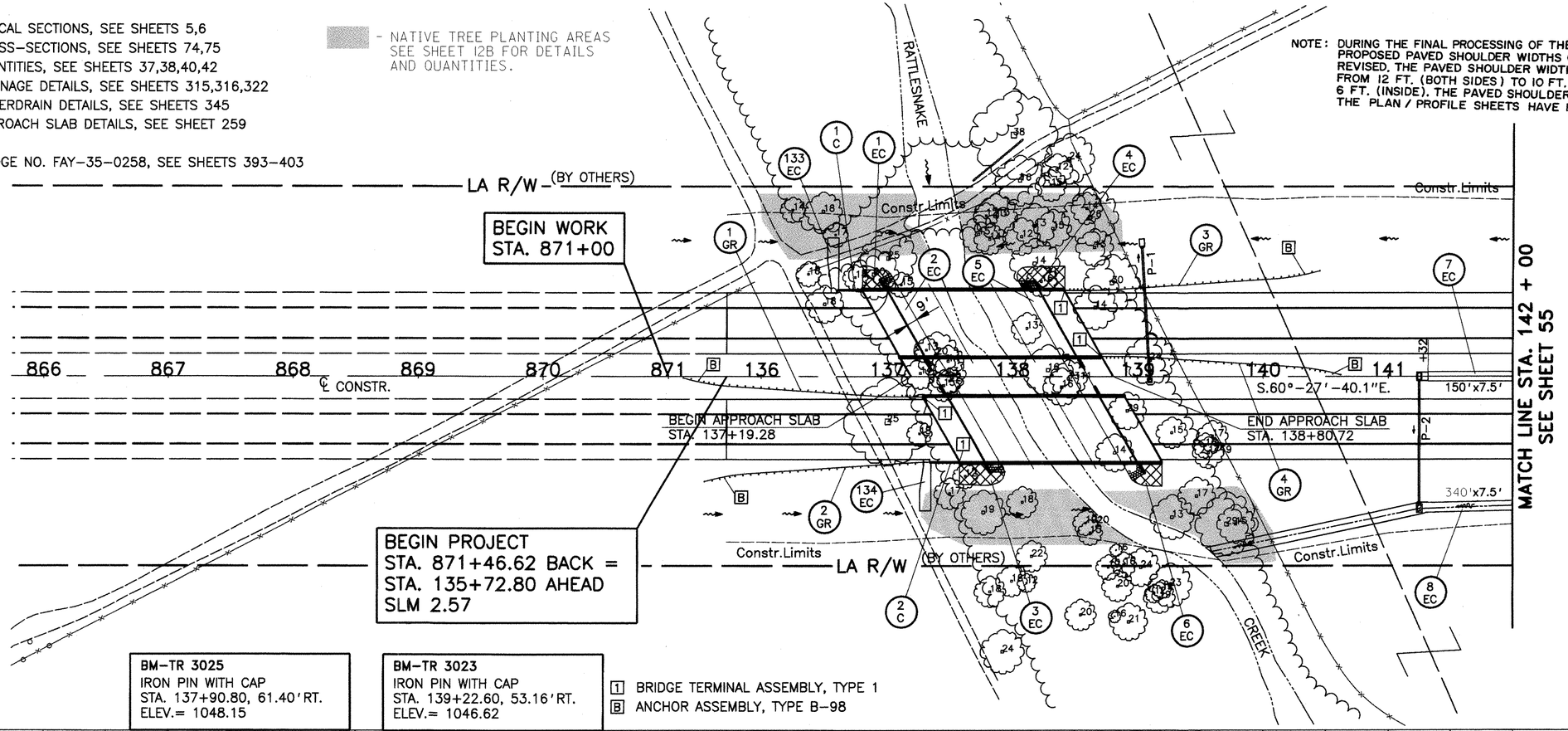
i:\pr\project\fay35\fay-35-2.57\ 06912dea.dgn

FOR TYPICAL SECTIONS, SEE SHEETS 5,6
FOR CROSS-SECTIONS, SEE SHEETS 74,75
FOR QUANTITIES, SEE SHEETS 37,38,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 315,316,322
FOR UNDERDRAIN DETAILS, SEE SHEETS 345
FOR APPROACH SLAB DETAILS, SEE SHEET 259

FOR BRIDGE NO. FAY-35-0258, SEE SHEETS 393-403

- NATIVE TREE PLANTING AREAS SEE SHEET 12B FOR DETAILS AND QUANTITIES.

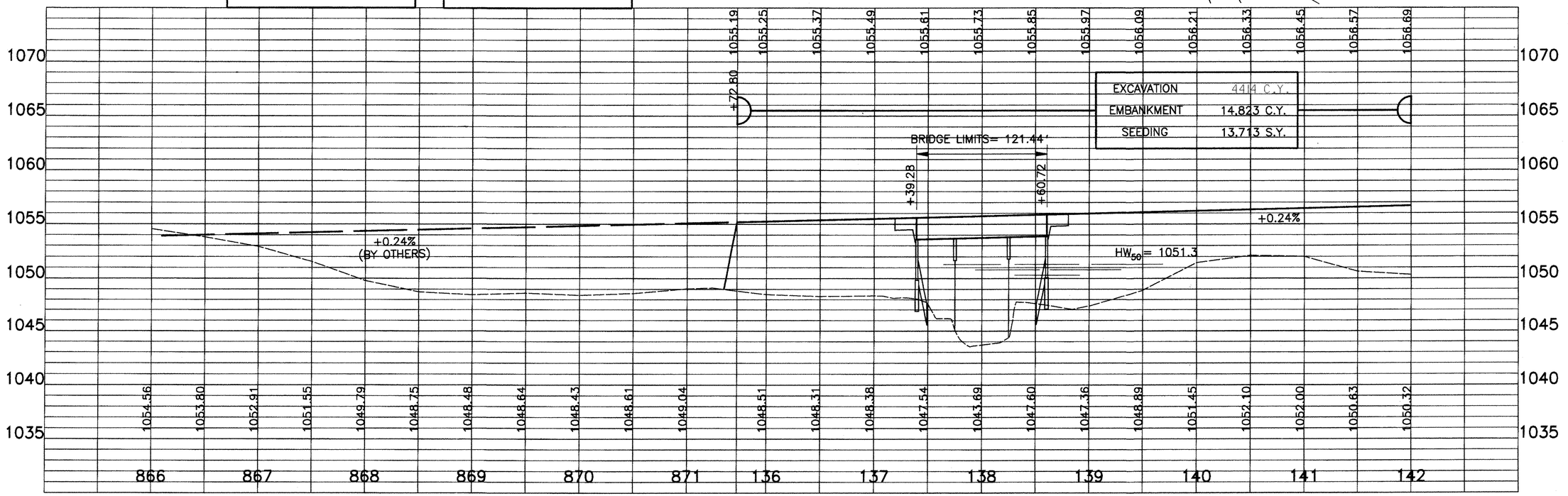
NOTE: DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE PAVED SHOULDER DIMENSIONS ON THE PLAN / PROFILE SHEETS HAVE NOT BEEN REVISED.



BM-TR 3025
IRON PIN WITH CAP
STA. 137+90.80, 61.40' RT.
ELEV.= 1048.15

BM-TR 3023
IRON PIN WITH CAP
STA. 139+22.60, 53.16' RT.
ELEV.= 1046.62

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[B] ANCHOR ASSEMBLY, TYPE B-98



EXCAVATION	4414 C.Y.
EMBANKMENT	14,823 C.Y.
SEEDING	13,713 S.Y.

REVISED 1-31-01 DSW

REVISED 10-99 BY FYC
CALCULATED
CHECKED

HORIZONTAL SCALE IN FEET 0 25 50 100

PLAN & PROFILE PROP. U.S.R. 35
STA. 866 + 00 BK. TO STA. 142 + 00 AHD.

FAY-35-2.57

54
522

i:\users\dwise\foy\35\257&75\40newsht\new054.dgn 20-MAR-2001 5:10PM dwise

SEE SHEET 54
MATCH LINE STA. 142 + 00

MATCH LINE STA. 156 + 00
SEE SHEET 56

STA.142+91
127'LT.

STA.142+87
118'LT.

15"TILE

10'-15"CONDUIT
TYPE F

1 D
9 EC
10 EC
7 EC
11 EC
12 EC
13 EC
14 EC
15 EC
8 EC

LA R/W (BY OTHERS)

Constr.Limits

170'x11'x1.5'
40'x10'
600'x17'
110'x12'
150'x7.5'
340'x7.5'

143 144 145 146 147 148 149 150 151 152 153 154 155

℄ CONSTR.

S.60°-27'-40.1"E

P-4
P-8
P-9
P-10
P-11
P-12

PVC SLEEVE (BY OTHERS)

15"TILE(ABANDON)

EX.15"TILE (ABANDON)

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS-SECTIONS, SEE SHEETS 75-78
FOR QUANTITIES, SEE SHEETS 37,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 315,316
FOR UNDERDRAIN DETAILS, SEE SHEET 345

EXCAVATION	12,109 C.Y.
EMBANKMENT	7760 C.Y.
SEEDING	31,820 S.Y.

+0.24%

1070 1065 1060 1055 1050 1045 1040 1035

142 143 144 145 146 147 148 149 150 151 152 153 154 155 156

HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED | CHECKED

REVISED 1/00 BY FYC

PLAN & PROFILE PROP. U.S.R. 35
STA. 142 + 00 TO STA. 156 + 00

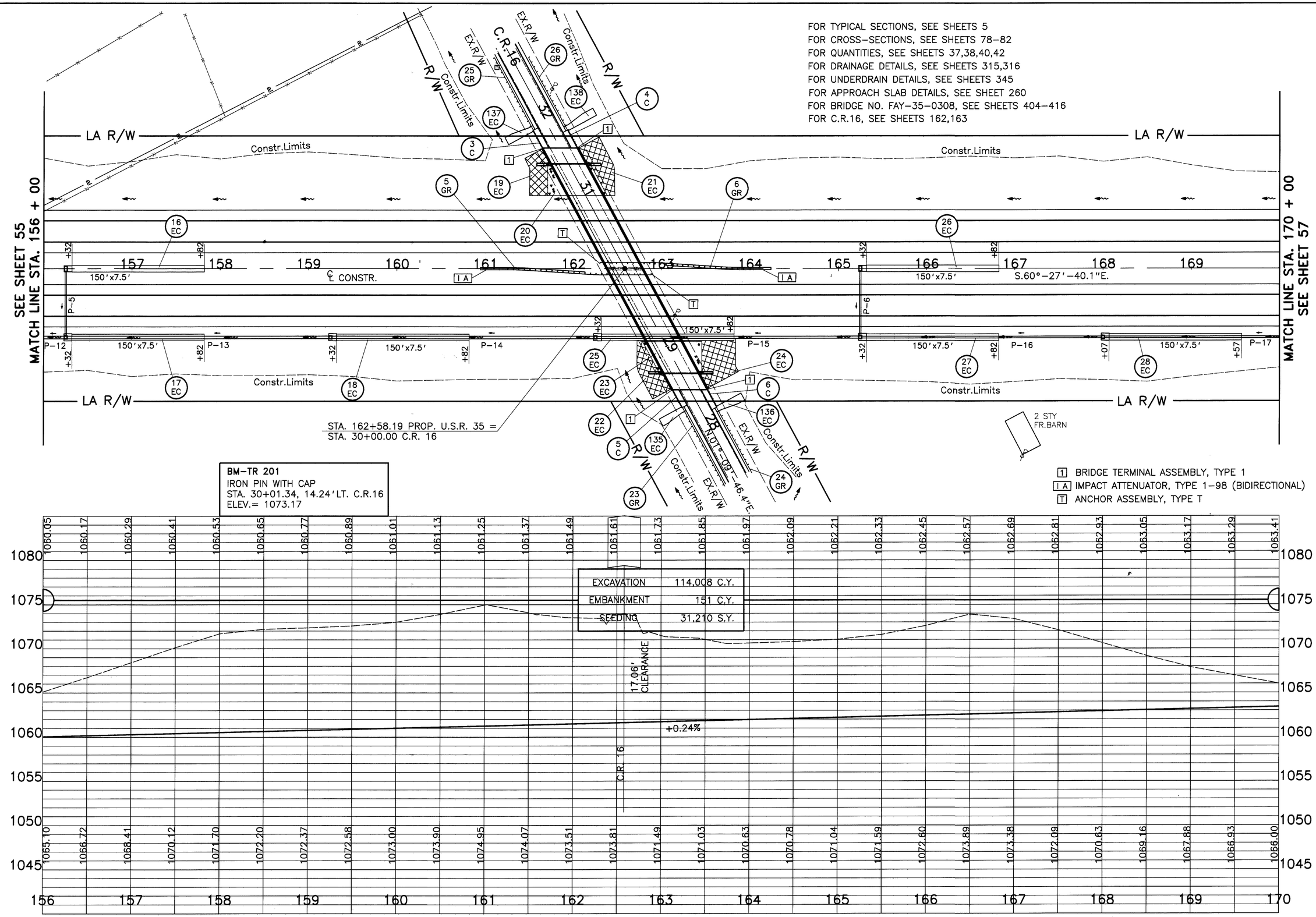
FAY-35-2.57

55
522

REVISED 1-31-01 DSW

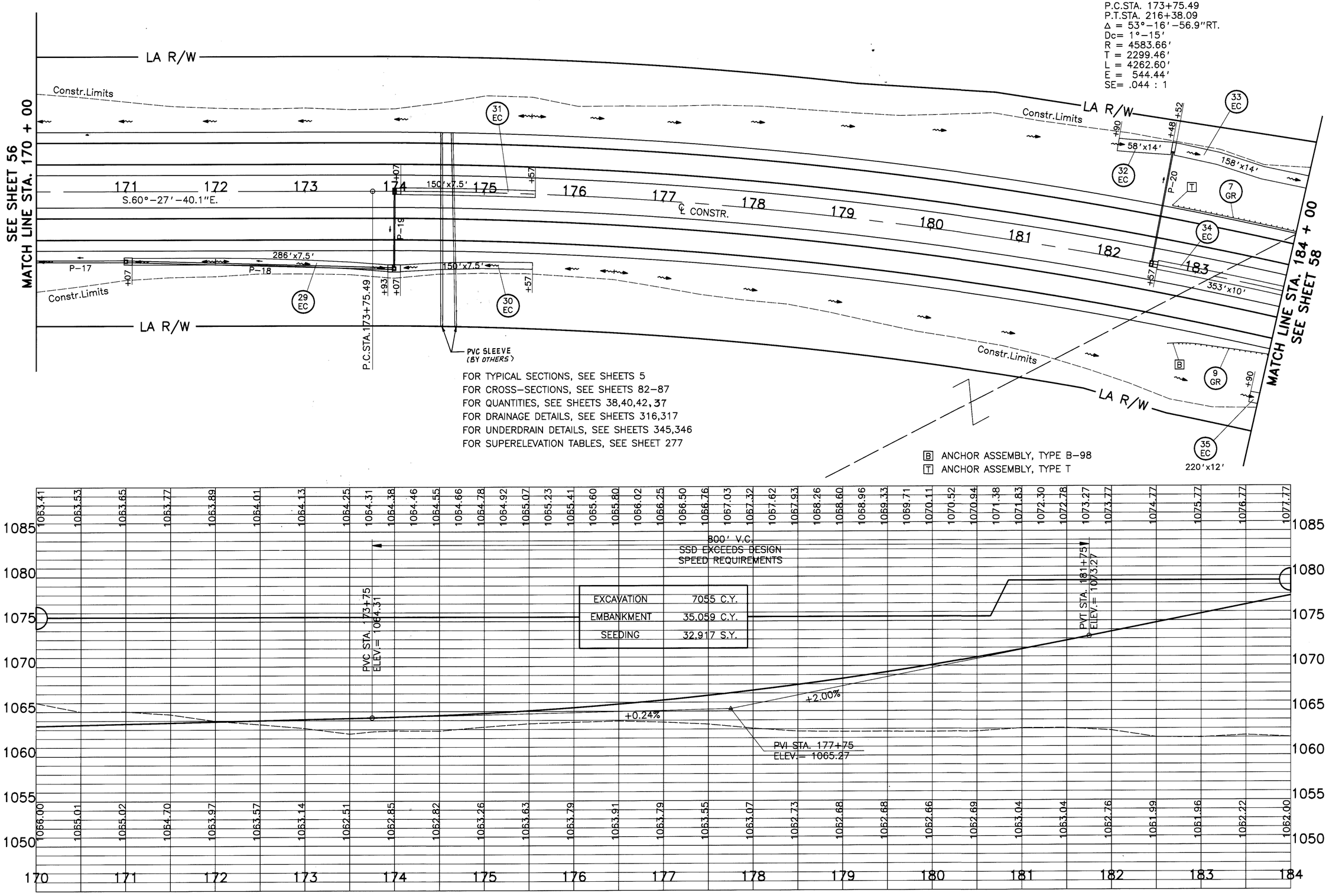
i:\users\dwise\fay\35\257&75\40lnewsht\new055.dgn 20-MAR-2001 4:12PM dwise

FOR TYPICAL SECTIONS, SEE SHEETS 5
FOR CROSS-SECTIONS, SEE SHEETS 78-82
FOR QUANTITIES, SEE SHEETS 37,38,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 315,316
FOR UNDERDRAIN DETAILS, SEE SHEETS 345
FOR APPROACH SLAB DETAILS, SEE SHEET 260
FOR BRIDGE NO. FAY-35-0308, SEE SHEETS 404-416
FOR C.R.16, SEE SHEETS 162,163



PLAN & PROFILE PROP. U.S.R. 35
STA. 156 + 00 TO STA. 170 + 00

FAY-35-2.57



HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED | CHECKED

REVISED 1/00 BY FYC

PLAN & PROFILE PROP. U.S.R. 35
STA. 170 + 00 TO STA. 184 + 00

FAY-35-2.57

BM-TR 203
CENTERLINE MONUMENT
STA. 400+00, CL, I-71
ELEV.= 1065.48

CURVE DATA
P.I.STA. 196+74.95
P.C.STA. 173+75.49
P.T.STA. 216+38.09
Δ = 53°-16'-56.9"RT.
Dc= 1°-15'
R = 4583.66'
T = 2299.46'
L = 4262.60'
E = 544.44'
SE= .044 : 1

STA. 194+83.12 PROP. U.S.R. 35 =
STA. 400+30.91 I-71

158'x14'
353'x10'
220'x12'
+10

LA R/W
Constr.Limits
EX.LA R/W
I-71
403
402
401
400
399
N.54°-59'-1.9"E.

BEGIN APPROACH SLAB
STA. 193+30.11
END APPROACH SLAB
STA. 196+36.13
℄ CONSTR.
S-2
S-3
P-46

PVI STA. 195+00
ELEV.= 1099.77
+2.00%
-2.00%
BRIDGE LIMITS = 266.02'
+50.11
+16.13

RETENTION POND

SEE SHEET 57
MATCH LINE STA. 184 + 00

MATCH LINE STA. 198 + 00
SEE SHEET 59

FOR APPROACH SLAB DETAILS, SEE SHEET 261
FOR I-71, SEE SHEET 177
FOR BRIDGE NO. FAY-35-0366, SEE SHEETS 417-436

FOR TYPICAL SECTIONS, SEE SHEETS 5,6
FOR CROSS-SECTIONS, SEE SHEETS 87-92
FOR QUANTITIES, SEE SHEETS 37,38,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 287,288,322
FOR UNDERDRAIN DETAILS, SEE SHEET 346
FOR SUPERELEVATION TABLES, SEE SHEET 277

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[2] BRIDGE TERMINAL ASSEMBLY, TYPE 2
[B] ANCHOR ASSEMBLY, TYPE B-98

EXCAVATION	836 C.Y.
EMBANKMENT	194,492 C.Y.
SEEDING	32,484 S.Y.

PVC STA. 84+00
ELEV.= 1077.77

17.30' CLEARANCE
17.33' CLEARANCE
2200' V.C.
SSD = 355

HORIZONTAL SCALE IN FEET: 0, 25, 50, 100
CALCULATED
CHECKED

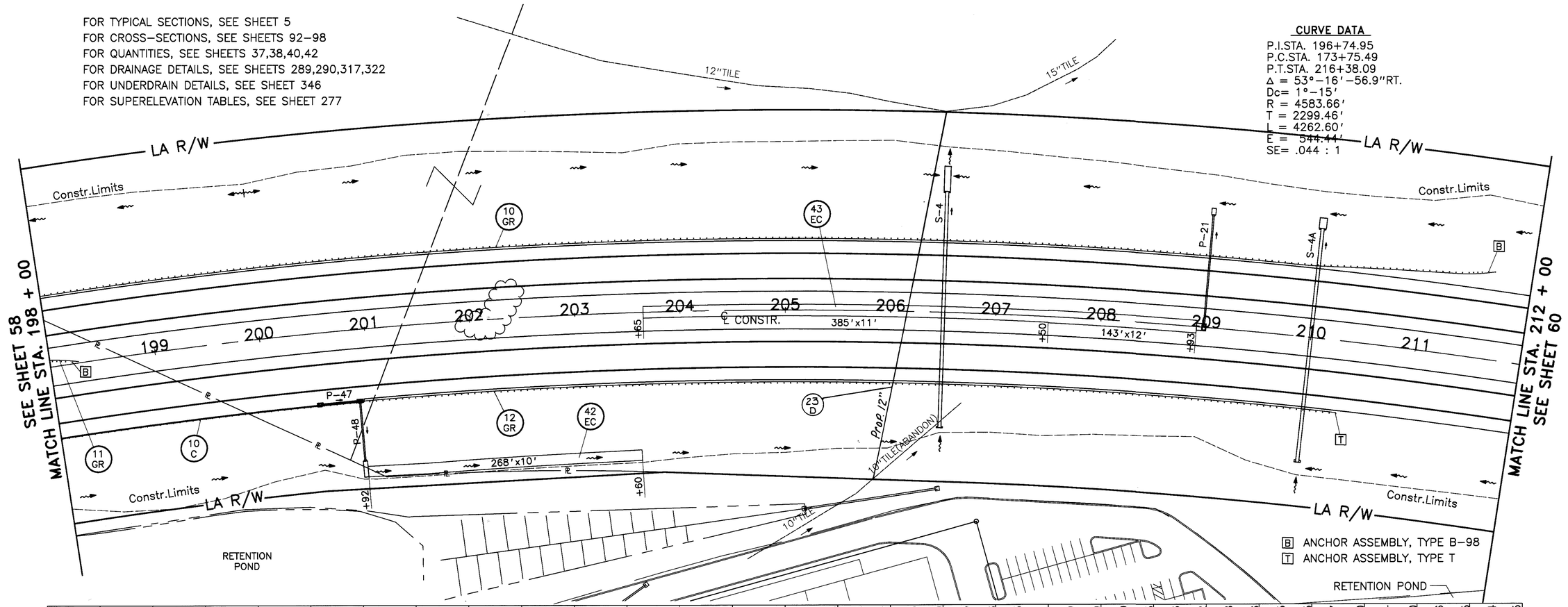
PLAN & PROFILE PROP. U.S.R. 35
STA. 184 + 00 TO STA. 198 + 00

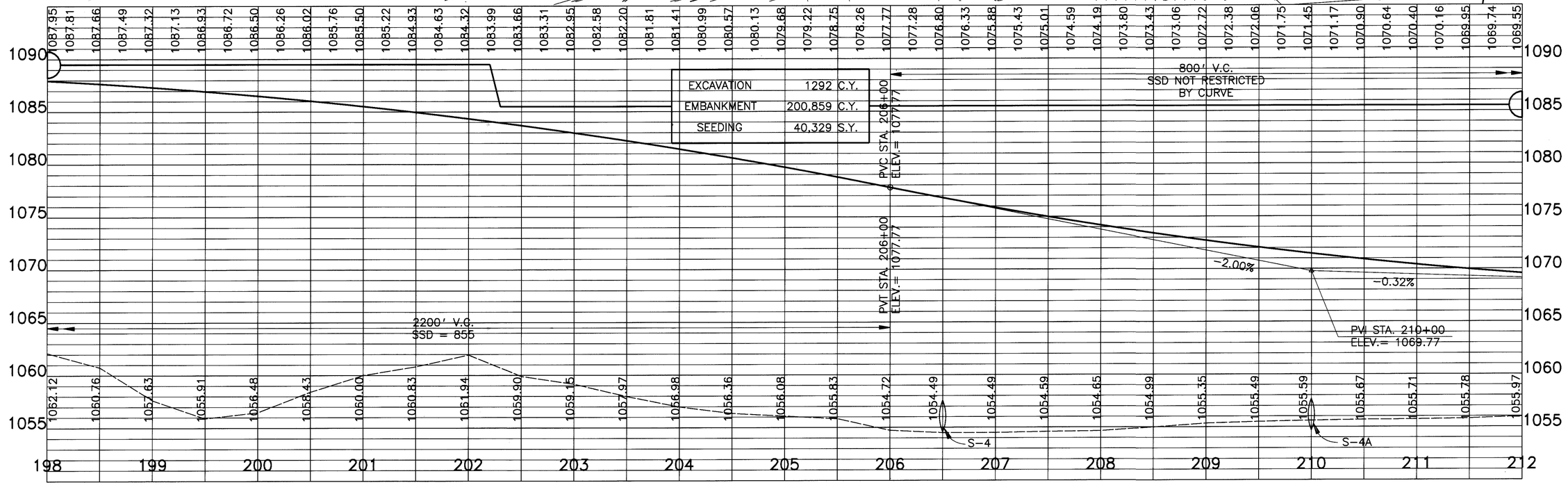
FAY-35-2.57

58
522

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS–SECTIONS, SEE SHEETS 92–98
FOR QUANTITIES, SEE SHEETS 37,38,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 289,290,317,322
FOR UNDERDRAIN DETAILS, SEE SHEET 346
FOR SUPERELEVATION TABLES, SEE SHEET 277

CURVE DATA
P.I.STA. 196+74.95
P.C.STA. 173+75.49
P.T.STA. 216+38.09
Δ = 53°–16′–56.9″RT.
Dc= 1°–15′
R = 4583.66′
T = 2299.46′
L = 4262.60′
E = 544.44′
SE= .044 : 1





PLAN & PROFILE PROP. U.S.R. 35
STA. 198 + 00 TO STA. 212 + 00

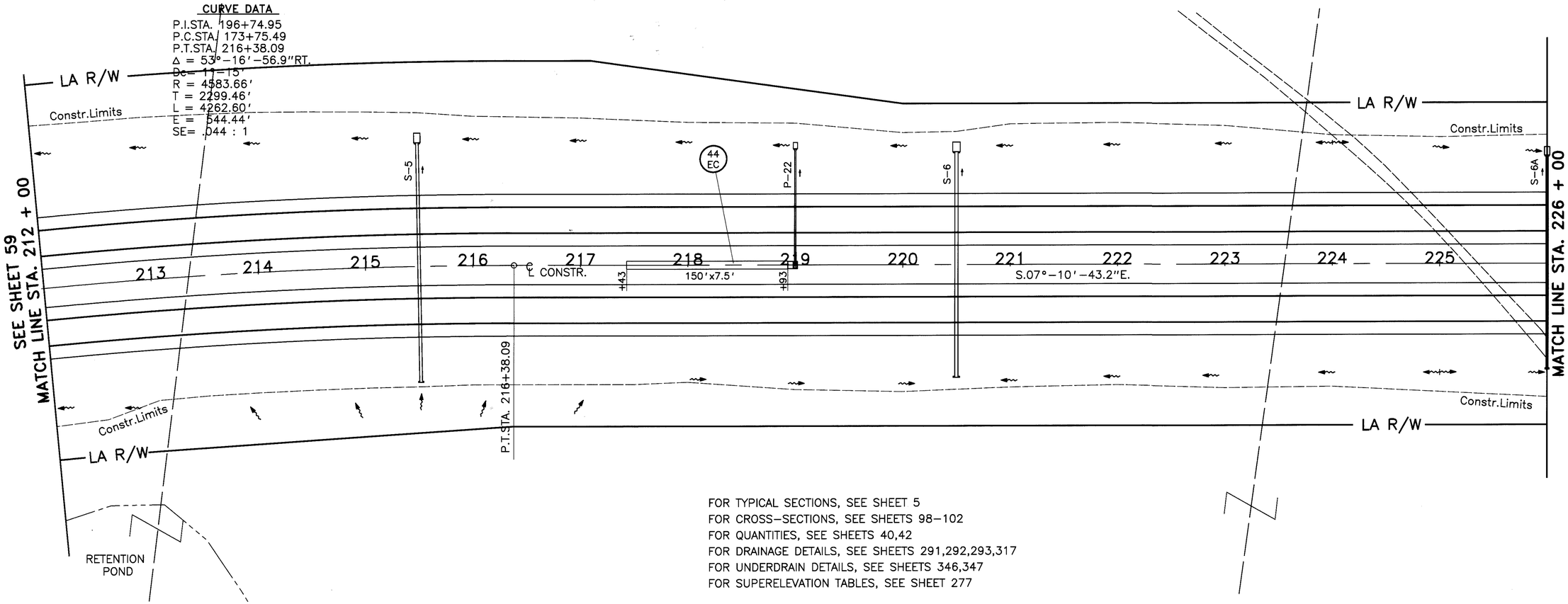
CALCULATED
CHECKED

HORIZONTAL SCALE IN FEET
0 25 50 100

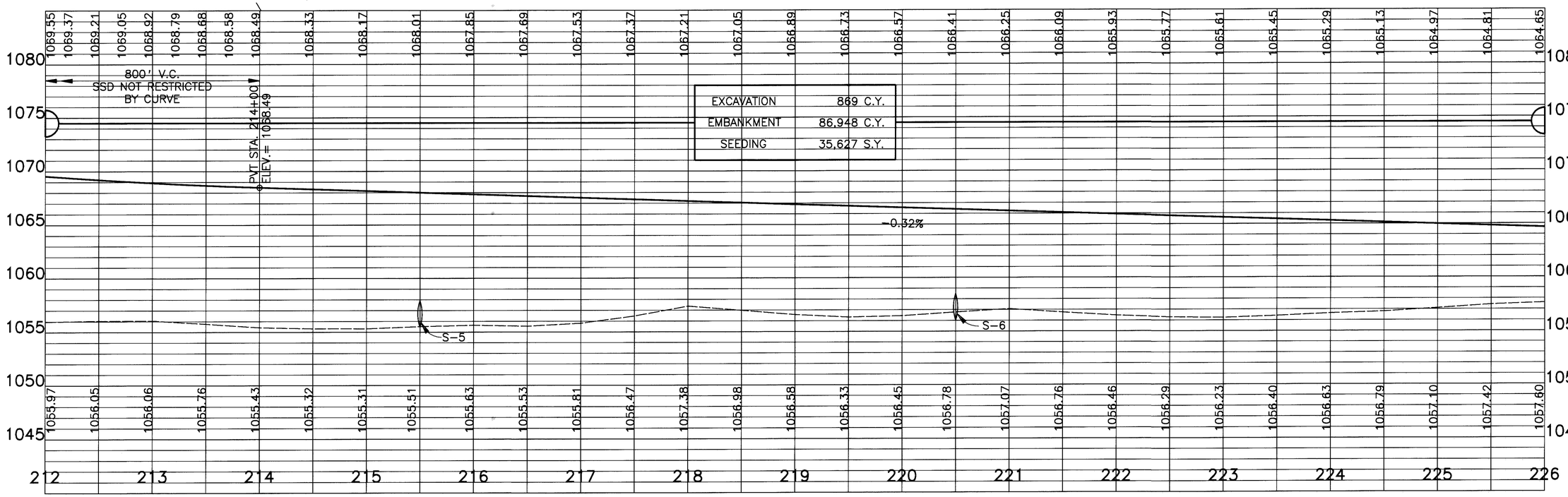
FAY–35–2.57

CURVE DATA

P.I.STA. 196+74.95
P.C.STA. 173+75.49
P.T.STA. 216+38.09
Δ = 53°–16′–56.9″RT.
Dc= 1°–15′
R = 4583.66′
T = 2299.46′
L = 4262.60′
E = 644.44′
SE= .044 : 1



FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS–SECTIONS, SEE SHEETS 98–102
FOR QUANTITIES, SEE SHEETS 40,42
FOR DRAINAGE DETAILS, SEE SHEETS 291,292,293,317
FOR UNDERDRAIN DETAILS, SEE SHEETS 346,347
FOR SUPERELEVATION TABLES, SEE SHEET 277



EXCAVATION	869 C.Y.
EMBANKMENT	86,948 C.Y.
SEEDING	35,627 S.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 212 + 00 TO STA. 226 + 00

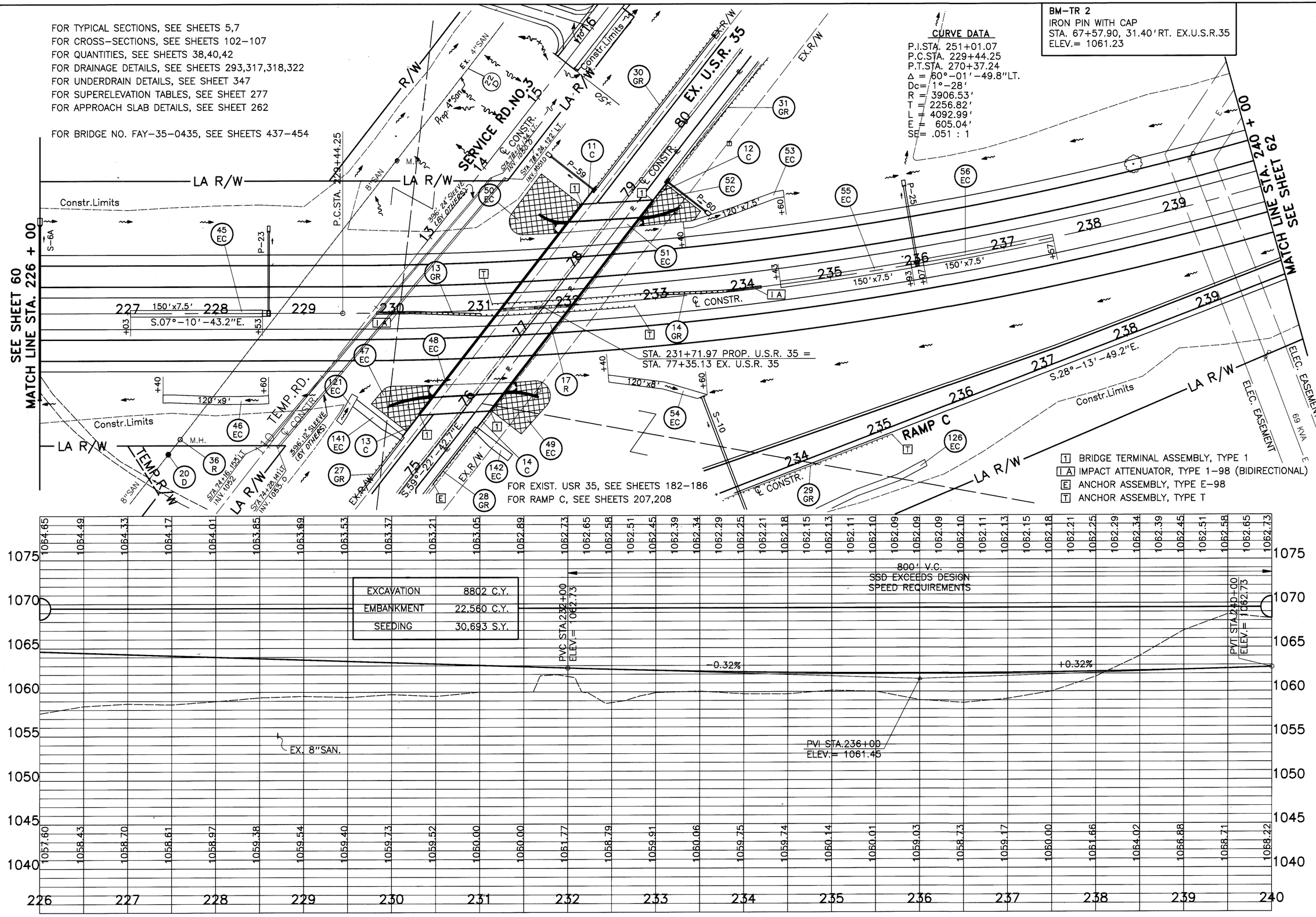
FAY–35–2.57

FOR TYPICAL SECTIONS, SEE SHEETS 5,7
FOR CROSS-SECTIONS, SEE SHEETS 102-107
FOR QUANTITIES, SEE SHEETS 38,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 293,317,318,322
FOR UNDERDRAIN DETAILS, SEE SHEET 347
FOR SUPERELEVATION TABLES, SEE SHEET 277
FOR APPROACH SLAB DETAILS, SEE SHEET 262

FOR BRIDGE NO. FAY-35-0435, SEE SHEETS 437-454

BM-TR 2
IRON PIN WITH CAP
STA. 67+57.90, 31.40' RT. EX.U.S.R.35
ELEV.= 1061.23

CURVE DATA
P.I.STA. 251+01.07
P.C.STA. 229+44.25
P.T.STA. 270+37.24
Δ = 60°-01'-49.8"LT.
Dc= 1°-28'
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'
SE= .051 : 1



EXCAVATION	8802 C.Y.
EMBANKMENT	22,560 C.Y.
SEEDING	30,693 S.Y.

HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED
CHECKED
REVISED 1/00
BY FYC

PLAN & PROFILE PROP. U.S.R. 35
STA. 226 + 00 TO STA. 240 + 00

FAY-35-2.57

FOR TYPICAL SECTIONS, SEE SHEETS 5,7
FOR CROSS-SECTIONS, SEE SHEETS 107-113
FOR QUANTITIES, SEE SHEETS 40,42,37
FOR UNDERDRAIN DETAILS, SEE SHEET 347
FOR PAVEMENT DETAILS, SEE SHEETS 270,271,273
FOR SUPERELEVATION TABLES, SEE SHEET 277

CURVE DATA
P.I.STA. 252+01.07
P.C.STA. 229+44.25
P.T.STA. 270+37.24
Δ = 60°-01'-49.8"LT.
Dc= 1°-28'
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'
SE= .051 : 1

SEE SHEET 61
MATCH LINE STA. 240 + 00

MATCH LINE STA. 254 + 00
SEE SHEET 63

ELEC. EASEMENT
69 KVA
ELEC. EASEMENT
Constr.Limits
LA R/W
RAMP D
S-11
WETLAND
57 EC
58 EC
59 EC
460'x14'
250'x15'
450'x16'
150'x7.5'
108'x10'
℄ CONSTR.
RAMP C
PVC SLEEVE (BY OTHERS)

FOR RAMP C, SEE SHEETS 207,208
FOR RAMP D, SEE SHEET 216

EXCAVATION	10,992 C.Y.
EMBANKMENT	56,152 C.Y.
SEEDING	28,858 S.Y.

PVI STA.240+00
ELEV.= 1062.73
+0.32%
PVI STA.247+00
ELEV.= 1064.97
-0.64%
PVC STA.243+00
ELEV.= 1063.69
PVT STA.251+00
ELEV.= 1062.41
800' V.C.
SSD EXCEEDS DESIGN SPEED REQUIREMENTS

REVISED 1/00 BY FYC
CALCULATED
CHECKED
HORIZONTAL SCALE IN FEET 0 25 50 100

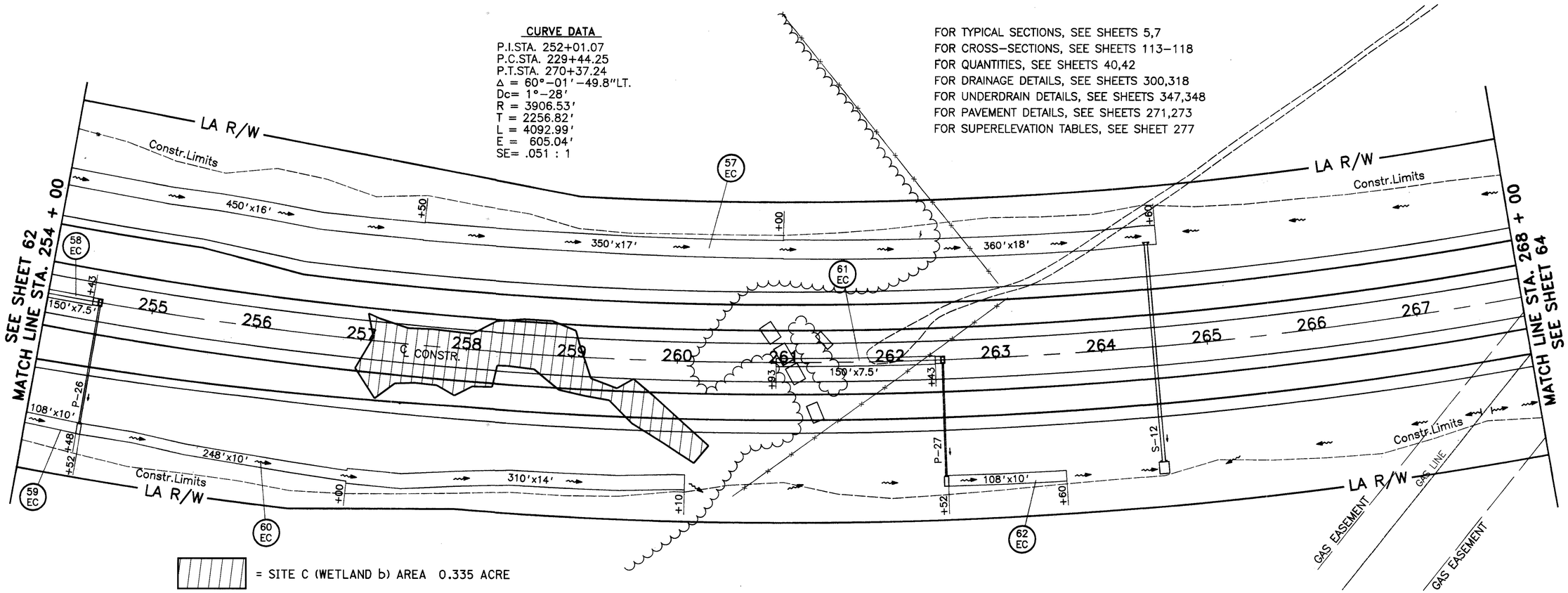
PLAN & PROFILE PROP. U.S.R. 35
STA. 240 + 00 TO STA. 254 + 00

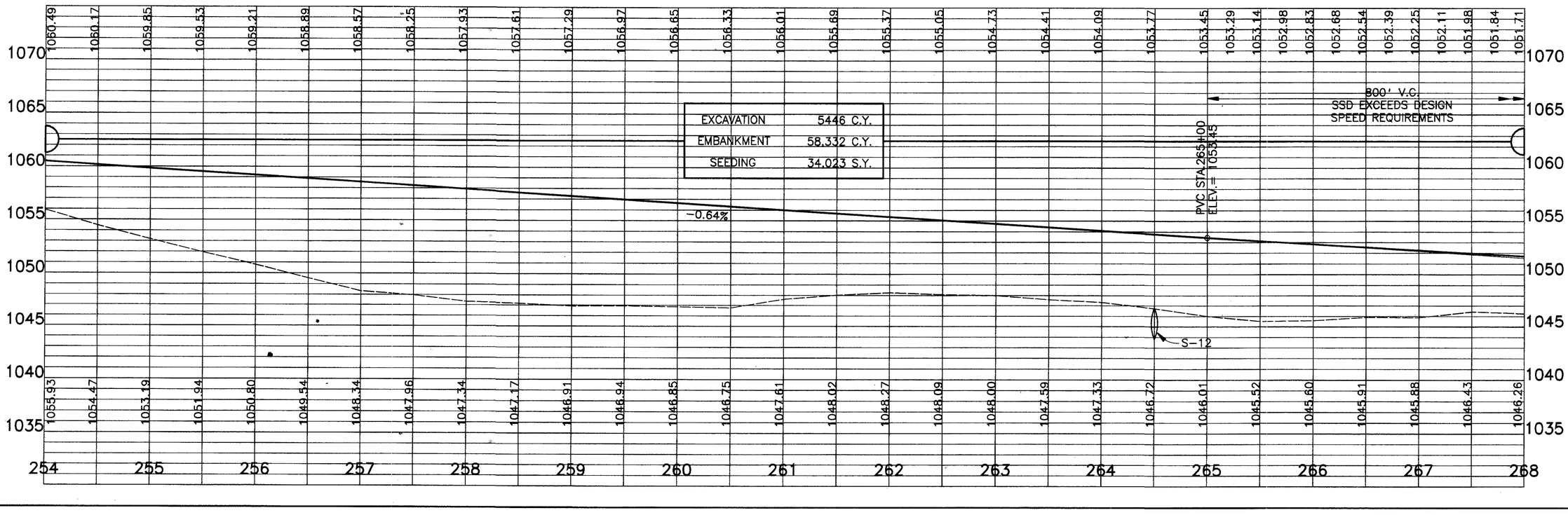
FAY-35-2.57

62
522

CURVE DATA
P.I.STA. 252+01.07
P.C.STA. 229+44.25
P.T.STA. 270+37.24
Δ = 60°−01′−49.8″LT.
Dc= 1°−28′
R = 3906.53′
T = 2256.82′
L = 4092.99′
E = 605.04′
SE= .051 : 1

FOR TYPICAL SECTIONS, SEE SHEETS 5,7
FOR CROSS−SECTIONS, SEE SHEETS 113−118
FOR QUANTITIES, SEE SHEETS 40,42
FOR DRAINAGE DETAILS, SEE SHEETS 300,318
FOR UNDERDRAIN DETAILS, SEE SHEETS 347,348
FOR PAVEMENT DETAILS, SEE SHEETS 271,273
FOR SUPERELEVATION TABLES, SEE SHEET 277



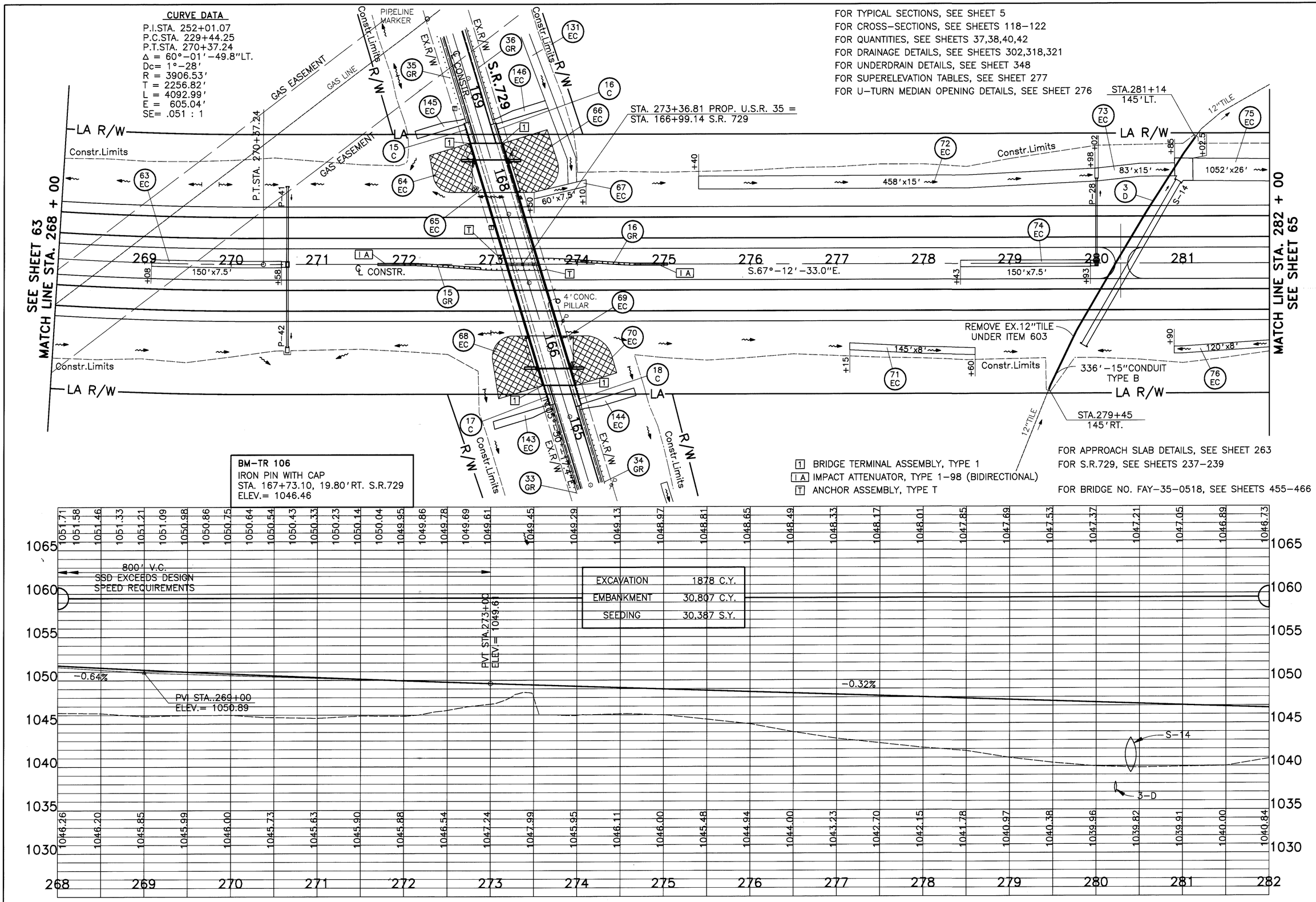


HORIZONTAL SCALE IN FEET 0 25 50 100
CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 254 + 00 TO STA. 268 + 00

FAY−35−2.57

63
522



EXCAVATION	1878 C.Y.
EMBANKMENT	30,807 C.Y.
SEEDING	30,387 S.Y.

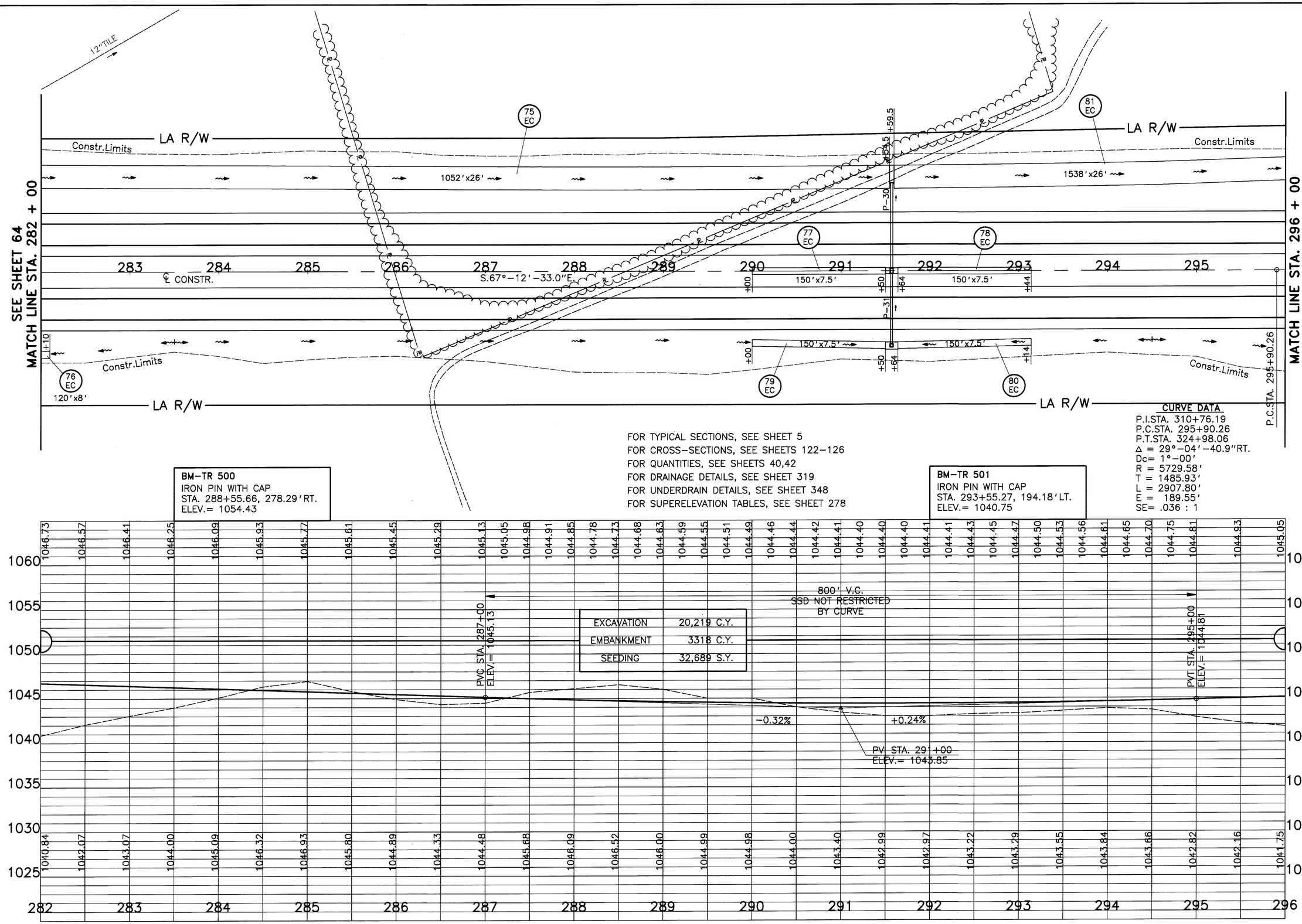
HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED

CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 268 + 00 TO STA. 282 + 00

FAY-35-2.57



HORIZONTAL SCALE IN FEET
0 25 50 100

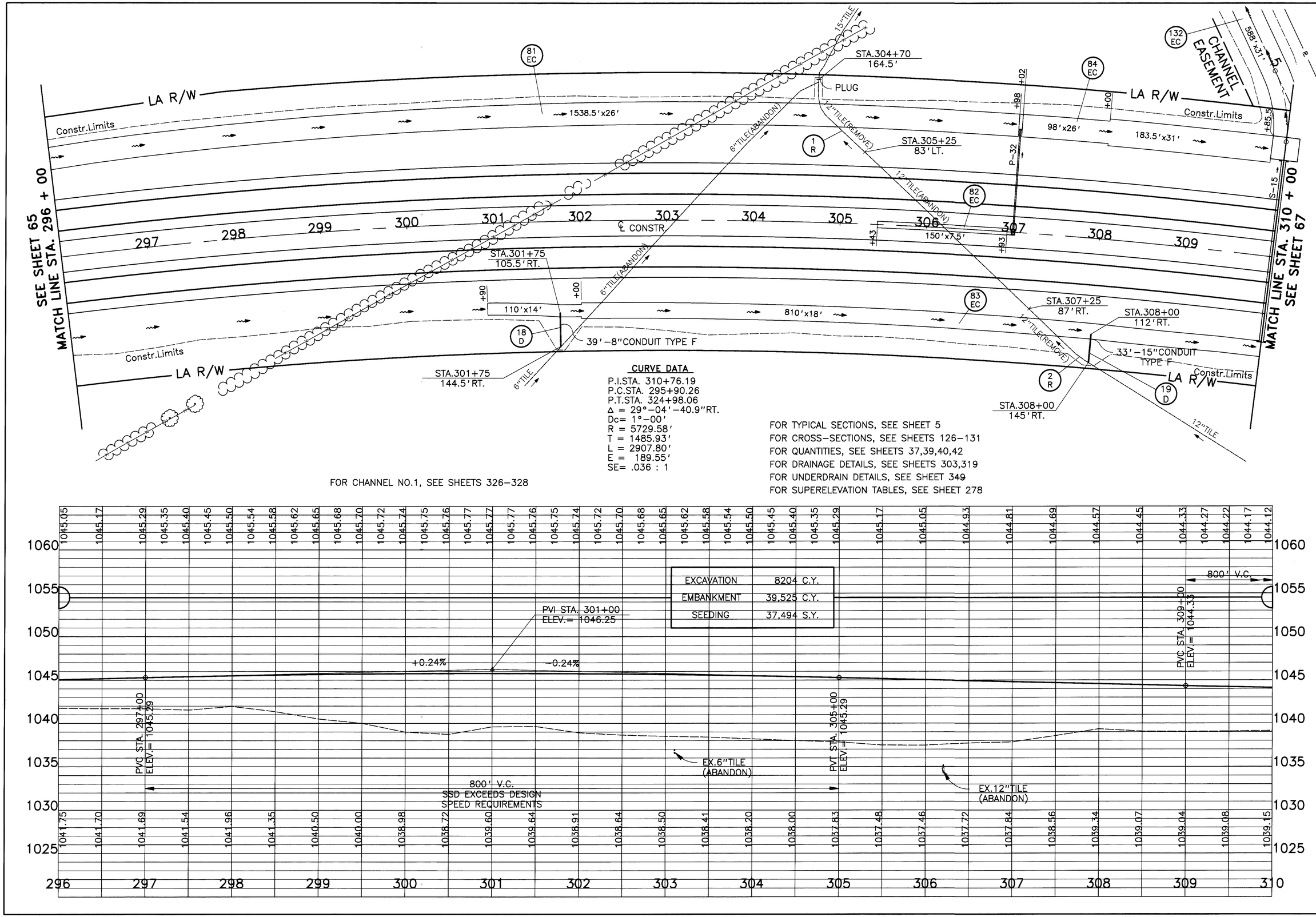
CALCULATED

CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 282+ 00 TO STA. 296 + 00

FAY-35-2.57

65
522



CURVE DATA

P.I.STA. 310+76.19
P.C.STA. 295+90.26
P.T.STA. 324+98.06
Δ = 29°-04'-40.9"RT.
Dc= 1°-00'
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'
SE= .036 : 1

FOR CHANNEL NO.1, SEE SHEETS 326-328

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS-SECTIONS, SEE SHEETS 126-131
FOR QUANTITIES, SEE SHEETS 37,39,40,42
FOR DRAINAGE DETAILS, SEE SHEETS 303,319
FOR UNDERDRAIN DETAILS, SEE SHEET 349
FOR SUPERELEVATION TABLES, SEE SHEET 278

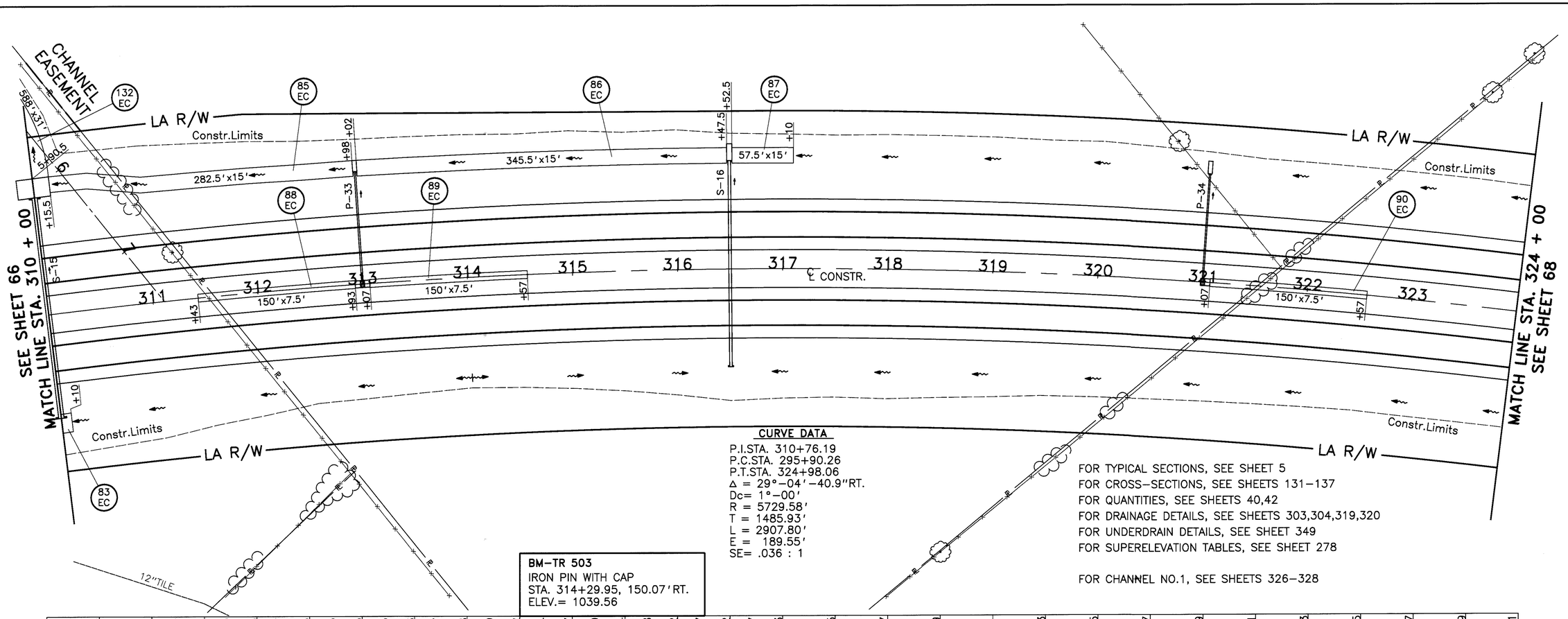
EXCAVATION	8204 C.Y.
EMBANKMENT	39,525 C.Y.
SEEDING	37,494 S.Y.

HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 296 + 00 TO STA. 310 + 00

FAY-35-2.57



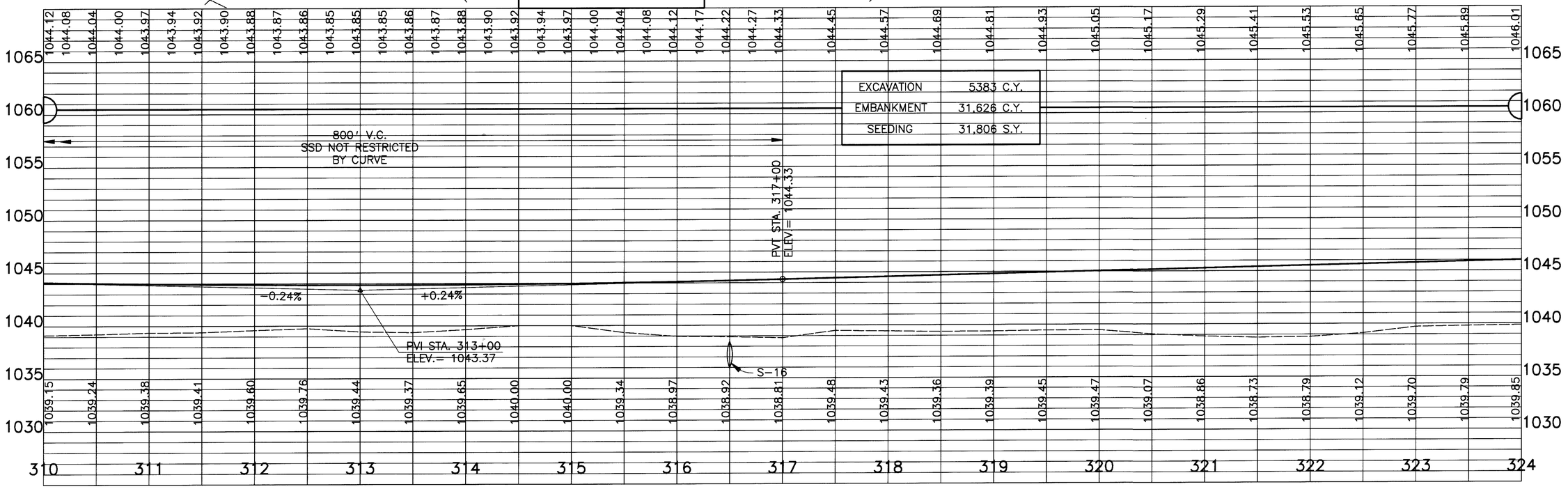
CURVE DATA

P.I.STA. 310+76.19
P.C.STA. 295+90.26
P.T.STA. 324+98.06
Δ = 29°–04'–40.9"RT.
Dc= 1°–00'
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'
SE= .036 : 1

BM–TR 503
IRON PIN WITH CAP
STA. 314+29.95, 150.07'RT.
ELEV.= 1039.56

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS–SECTIONS, SEE SHEETS 131–137
FOR QUANTITIES, SEE SHEETS 40,42
FOR DRAINAGE DETAILS, SEE SHEETS 303,304,319,320
FOR UNDERDRAIN DETAILS, SEE SHEET 349
FOR SUPERELEVATION TABLES, SEE SHEET 278

FOR CHANNEL NO.1, SEE SHEETS 326–328

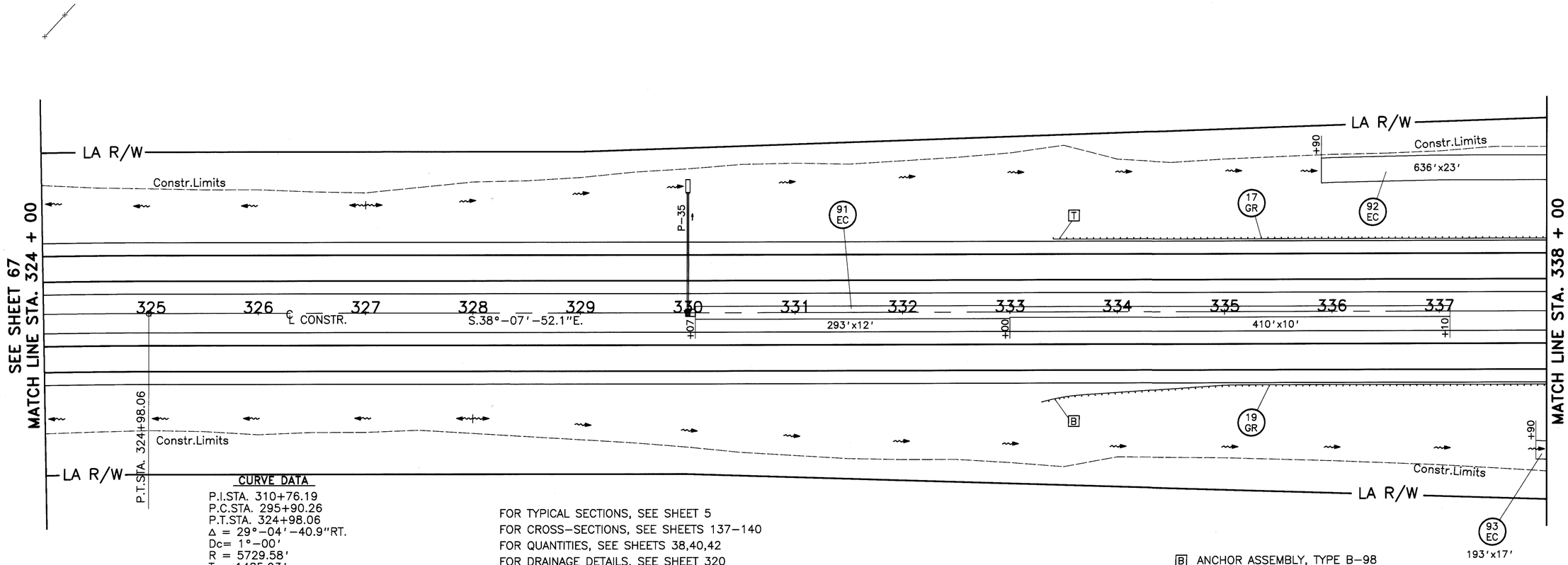


EXCAVATION	5383 C.Y.
EMBANKMENT	31,626 C.Y.
SEEDING	31,806 S.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100
CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 310 + 00 TO STA. 324 + 00

FAY–35–2.57

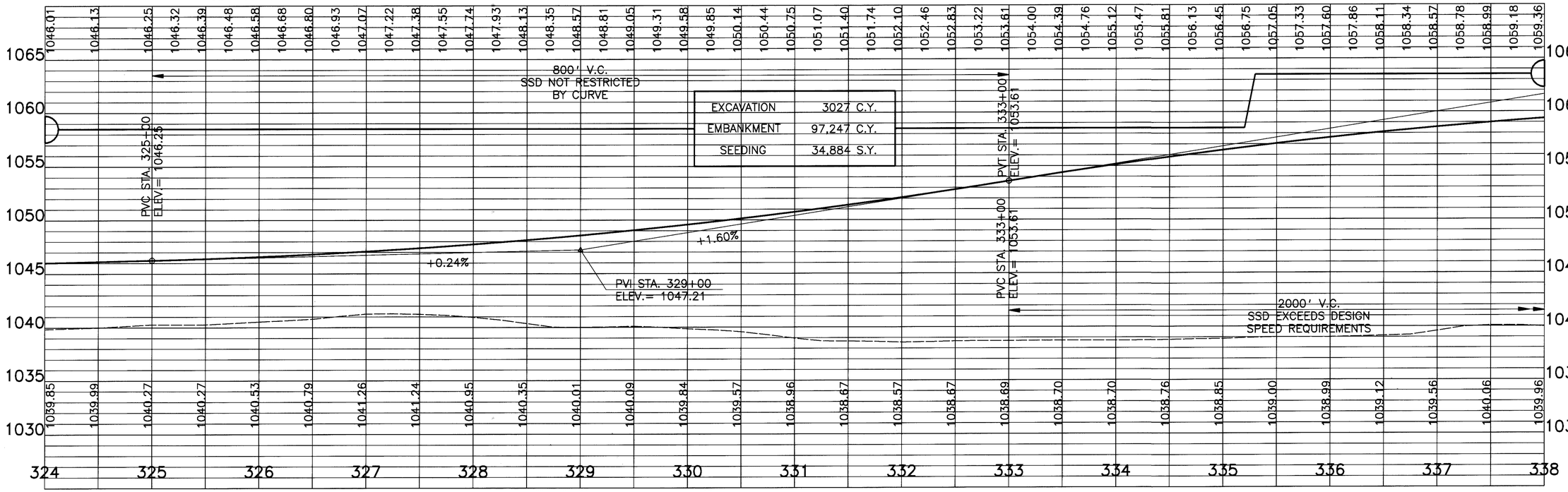


CURVE DATA

P.I.STA. 310+76.19
P.C.STA. 295+90.26
P.T.STA. 324+98.06
Δ = 29°-04'-40.9"RT.
Dc= 1°-00'
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'
SE= .036 : 1

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS-SECTIONS, SEE SHEETS 137-140
FOR QUANTITIES, SEE SHEETS 38,40,42
FOR DRAINAGE DETAILS, SEE SHEET 320
FOR UNDERDRAINS, SEE SHEETS 349,350
FOR SUPERELEVATION TABLES, SEE SHEET 278

[B] ANCHOR ASSEMBLY, TYPE B-98
[T] ANCHOR ASSEMBLY, TYPE T



EXCAVATION	3027 C.Y.
EMBANKMENT	97,247 C.Y.
SEEDING	34,884 S.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 324 + 00 TO STA. 338 + 00

FAY-35-2.57

BM-TR 101
IRON PIN WITH CAP
STA. 88+95.70, 7.50'RT. C.R.85
ELEV.= 1041.52

CURVE DATA
P.I.STA. 363+90.54
P.C.STA. 345+83.02
P.T.STA. 380+84.82
Δ = 35°-01'-04.9"LT.
Dc= 1°-00'
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'
SE= .036 : 1

SEE SHEET 68
MATCH LINE STA. 338 + 00

MATCH LINE STA. 352 + 00
SEE SHEET 70

STA. 341+47.44 PROP. U.S.R. 35 =
STA. 85+00.00 C.R. 85

LA R/W
Constr.Limits
EX.R/W
636'x23'
465'x28'
398'x31'
353'x11'
193'x17'
1025'x17'
1710'x20'
S.38°-07'-52.1"E.
S.85°-41'-02.0"E.
℄ CONSTR.
S-18
S-17
P.C.STA. 345+83.02
BEGIN APPROACH SLAB
STA. 340+44.92
END APPROACH SLAB
STA. 342+49.96
8" TILE(ABANDON)
10" TILE(ABANDON)
8" TILE(REMOVED)
8" TILE
35'-10"CONDUIT
TYPE F
STA.346+83
136'RT.
STA.346+83
171'RT.
STA.347+50
94'RT.
C.R.85

FOR TYPICAL SECTIONS, SEE SHEETS 5,6
FOR CROSS-SECTIONS, SEE SHEETS 140-145
FOR QUANTITIES, SEE SHEETS 37,38,39,40,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 305,306,320
FOR UNDERDRAIN DETAILS, SEE SHEET 350
FOR SUPERELEVATION TABLES, SEE SHEET 278

PVI STA. 343+00
ELEV.= 1069.61

FOR APPROACH SLAB DETAILS, SEE SHEET 264
FOR C.R.85 CROSS-SECTIONS, SEE SHEETS 255-258
FOR BRIDGE NO. FAY-35-0644, SEE SHEETS 467-482

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[2] BRIDGE TERMINAL ASSEMBLY, TYPE 2
[B] ANCHOR ASSEMBLY, TYPE B-98

+1.60%
-2.00%
BRIDGE LIMITS = 155.04'
+89.92
+24.96
16.07'
CLEARANCE
2000' V.C.
SSD EXCEEDS DESIGN
SPEED REQUIREMENTS

	MAINLINE	C.R.85	TOTAL
EXCAVATION	11,782 C.Y.	1233 C.Y.	13,015 C.Y.
EMBANKMENT	151,753 C.Y.	3741 C.Y.	155,494 C.Y.
SEEDING	40,940 S.Y.	5031 S.Y.	46,971 S.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100
CALCULATED
CHECKED

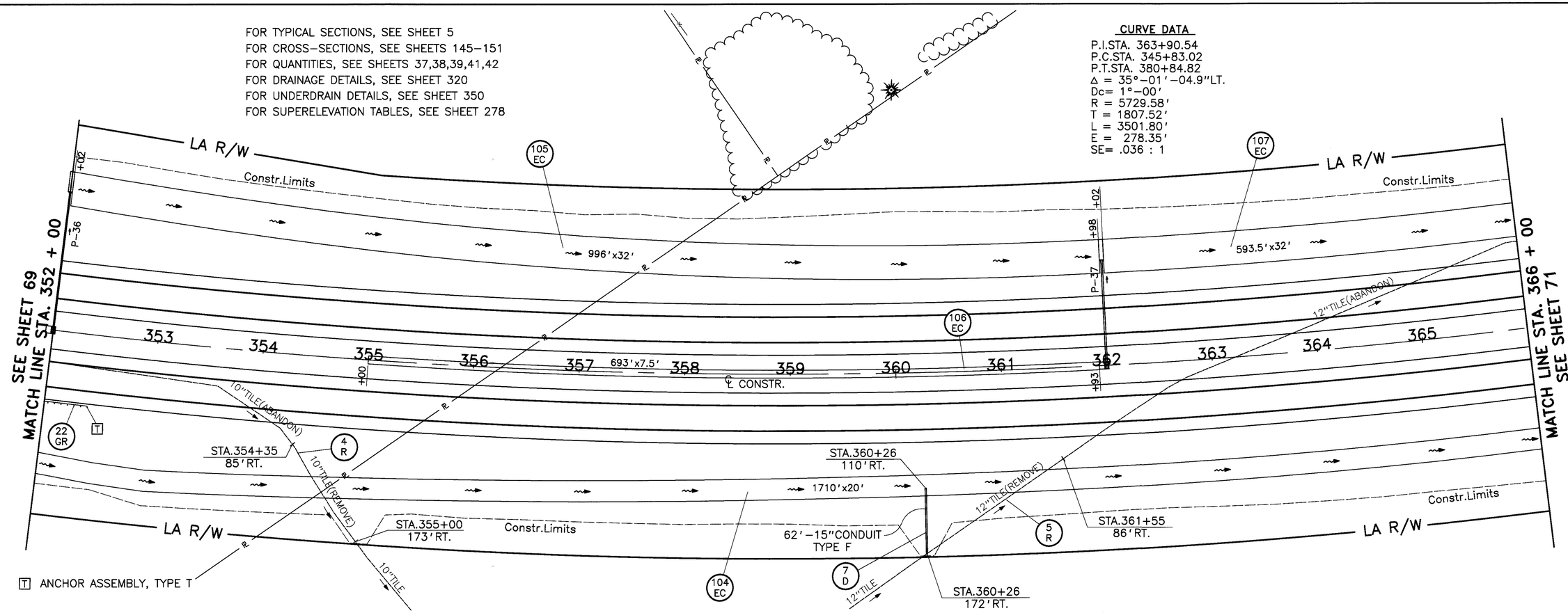
PLAN & PROFILE PROP. U.S.R. 35
STA. 338 + 00 TO STA. 352 + 00

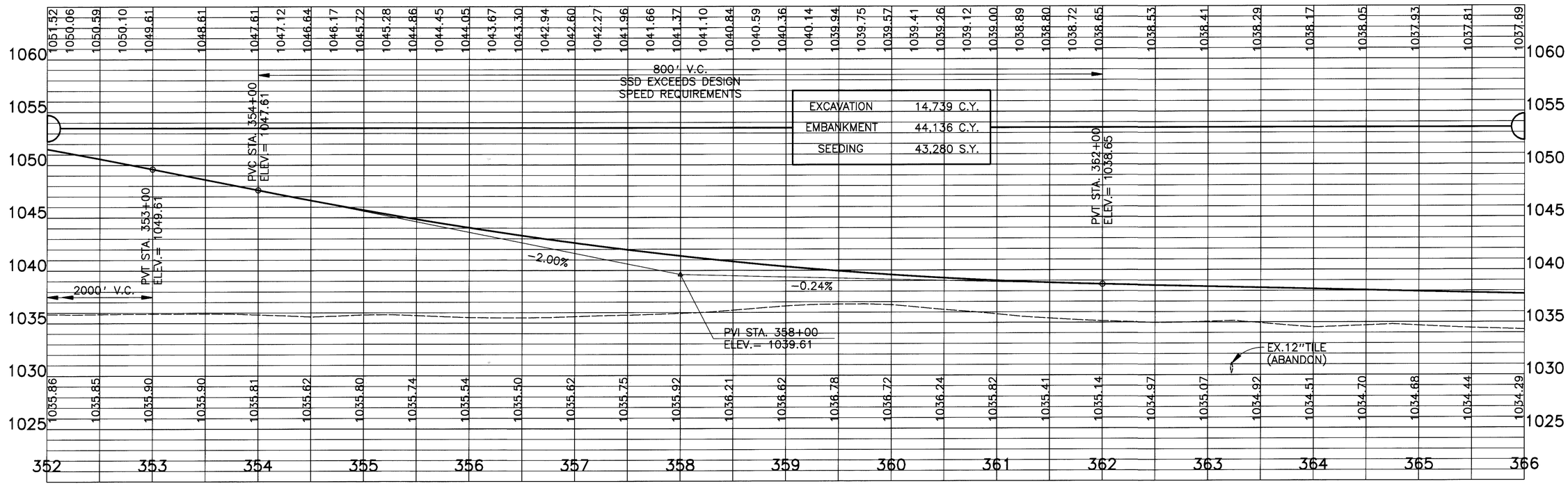
FAY-35-2.57

69
522

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS-SECTIONS, SEE SHEETS 145-151
FOR QUANTITIES, SEE SHEETS 37,38,39,41,42
FOR DRAINAGE DETAILS, SEE SHEET 320
FOR UNDERDRAIN DETAILS, SEE SHEET 350
FOR SUPERELEVATION TABLES, SEE SHEET 278

CURVE DATA
P.I.STA. 363+90.54
P.C.STA. 345+83.02
P.T.STA. 380+84.82
Δ = 35°-01'-04.9"LT.
Dc= 1°-00'
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'
SE= .036 : 1





PLAN & PROFILE PROP. U.S.R. 35
STA. 352 + 00 TO STA. 366 + 00

CALCULATED
CHECKED

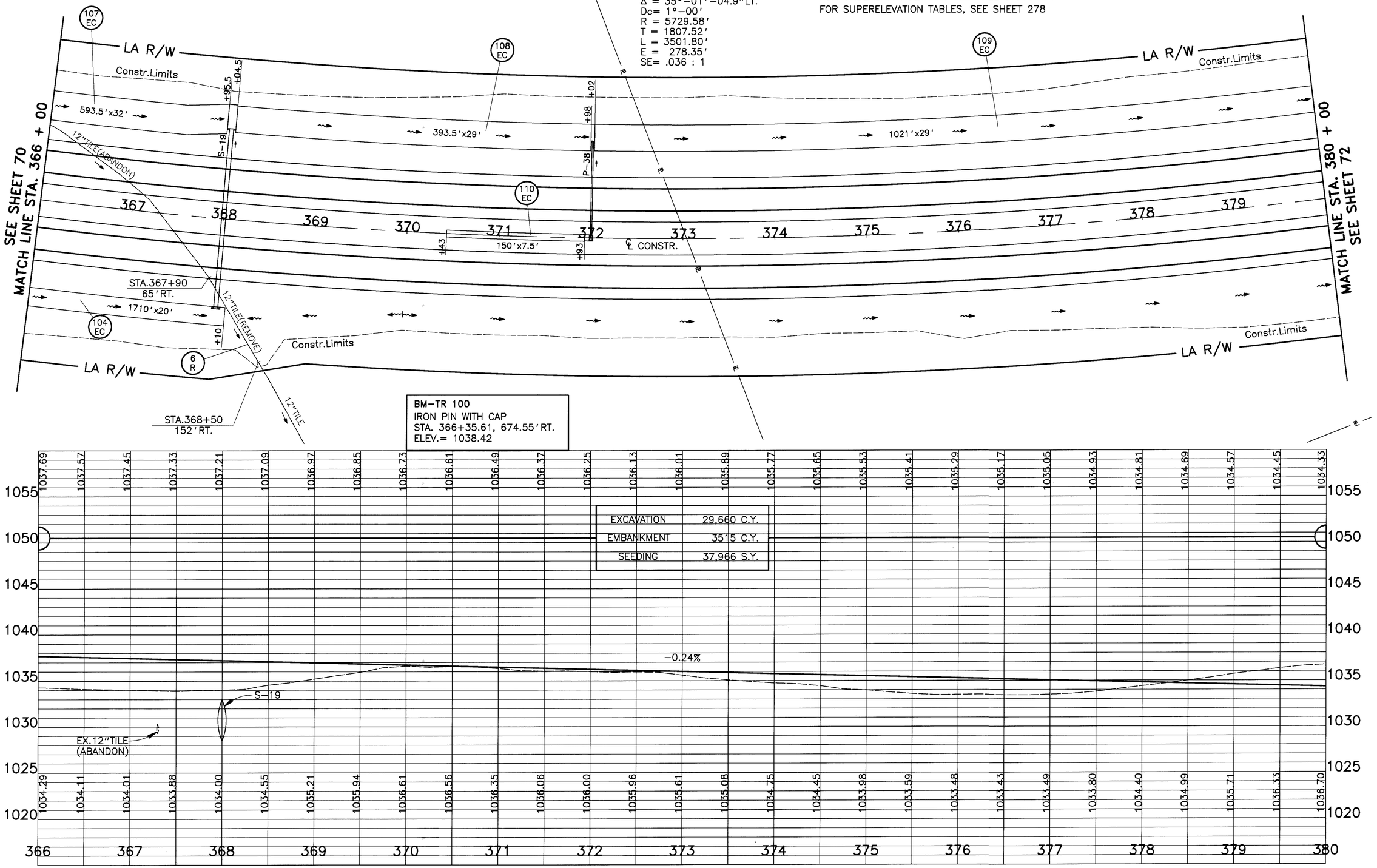
0 25 50 100 HORIZONTAL SCALE IN FEET

FAY-35-2.57

CURVE DATA

P.I.STA. 363+90.54
P.C.STA. 345+83.02
P.T.STA. 380+84.82
Δ = 35°–01′–04.9″LT.
Dc= 1°–00′
R = 5729.58′
T = 1807.52′
L = 3501.80′
E = 278.35′
SE= .036 : 1

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS–SECTIONS, SEE SHEETS 151–157
FOR QUANTITIES, SEE SHEETS 37,39,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 307,320
FOR UNDERDRAIN DETAILS, SEE SHEETS 350,351
FOR SUPERELEVATION TABLES, SEE SHEET 278



HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED

CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 366 + 00 TO STA. 380 + 00

FAY–35–2.57

71
522

CURVE DATA
P.I.STA. 363+90.54
P.C.STA. 345+83.02
P.T.STA. 380+84.82
Δ = 35°–01′–04.9″LT.
Dc= 1°–00′
R = 5729.58′
T = 1807.52′
L = 3501.80′
E = 278.35′
SE= .036 . 1

SEE SHEET 71
MATCH LINE STA. 380 + 00

MATCH LINE STA. 394 + 00
SEE SHEET 73

109 EC
112 EC
116 EC
117 EC
114 EC
115 EC
111 EC
113 EC
7 R
8 D

P.T.STA. 380+84.82
P–39
S–20
P–40
+23 +27
+69 +81
+16 +20

LA R/W
Constr.Limits
CHANNEL EASEMENT
1021′x29′
742′x29′
135′x35′
580′x35′
15″TILE(ABANDON)
6″TILE(ABANDON)
POLE BARN
℄ CONSTR.
S.73°–08′–57.0″E.
CONC. BLOCK FOUNDATION
WINDMILL
381 382 383 384 385 386 387 388 389 390 391 392 393
150′x7.5′
+68 +18
+61 +25
+111 +75
STA.388+90
108′RT.
135′x10′
85′x7.5′
+40 +75 +60
6″TILE
39′–6″CONDUIT TYPE F
STA.388+90
147′RT.
8″G 26″G 24″G 20″G
PIPELINE EASEMENT
(BY OTHERS)

FOR TYPICAL SECTIONS, SEE SHEET 5
FOR CROSS–SECTIONS, SEE SHEETS 157–161
FOR QUANTITIES, SEE SHEETS 37,39,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 308,321
FOR UNDERDRAIN DETAILS, SEE SHEET 351
FOR SUPERELEVATION TABLES, SEE SHEET 278
FOR U–TURN MEDIAN OPENING DETAILS, SEE SHEET 276

FOR CHANNEL NO.2, SEE SHEETS 329–332

EXCAVATION	46,039 C.Y.
EMBANKMENT	2015 C.Y.
SEEDING	34.527 S.Y.

PVC STA. 387+75
ELEV.= 1032.47
800′ V.C.
SSD NOT RESTRICTED BY CURVE
–0.24%
–0.24%
+0.32%
PVI STA. 391+75
ELEV.= 1031.51
S–20
EX.15″TILE (ABANDON)
EX.8″GAS
EX.26″GAS
EX.24″GAS

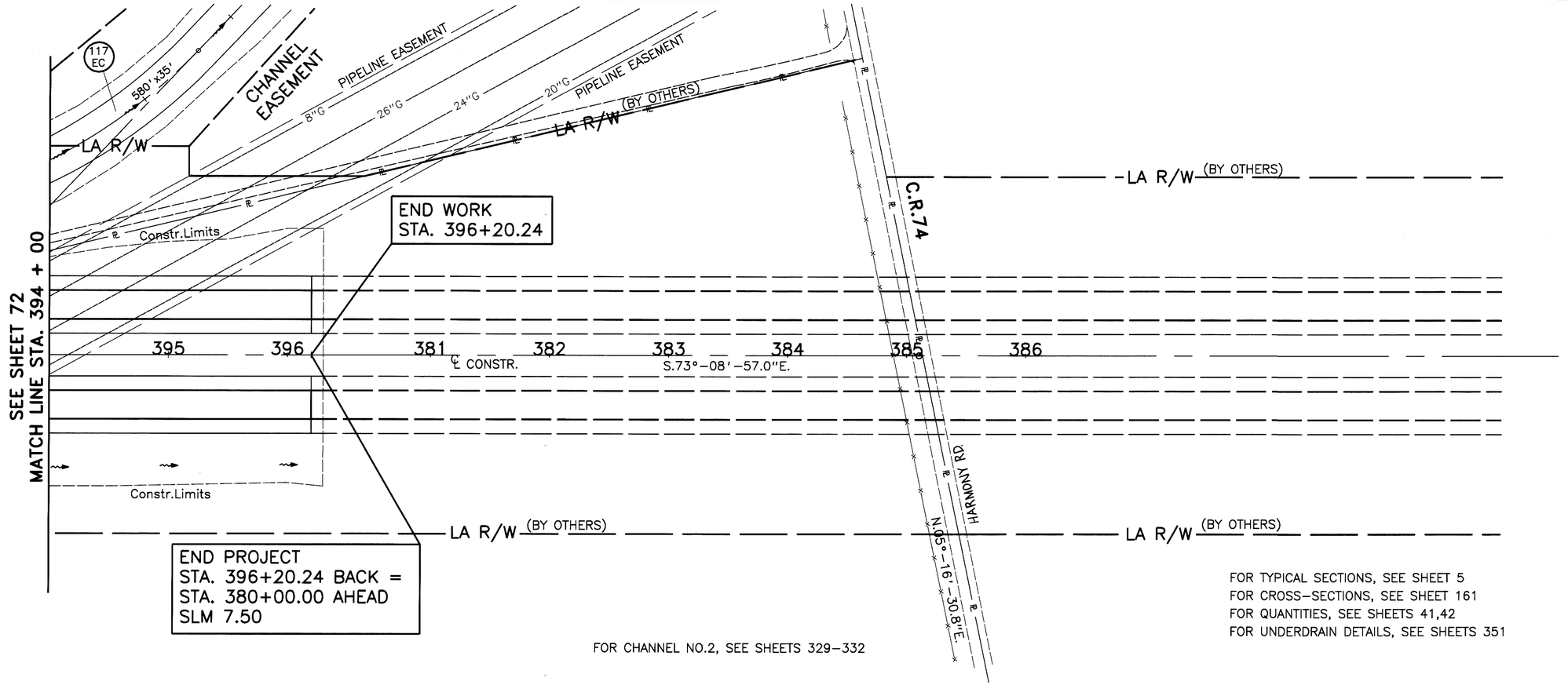
1050 1045 1040 1035 1030 1025 1020 1015
380 381 382 383 384 385 386 387 388 389 390 391 392 393 394

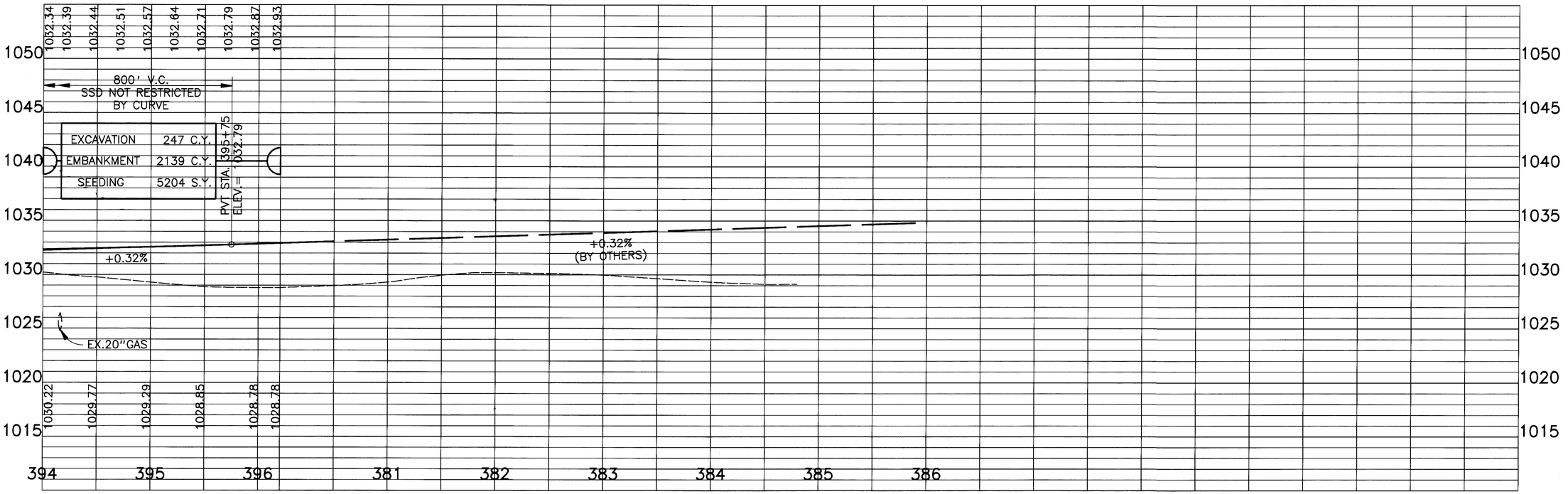
SCALE IN FEET: 0 25 50 100
CALCULATED
CHECKED

PLAN & PROFILE PROP. U.S.R. 35
STA. 380 + 00 TO STA. 394 + 00

FAY–35–2.57

72
522





CALCULATED | CHECKED

HORIZONTAL SCALE IN FEET
0 25 50 100

PLAN & PROFILE PROP. U.S.R. 35
STA. 394 + 00 BK. TO STA. 386 + 00 AHD.

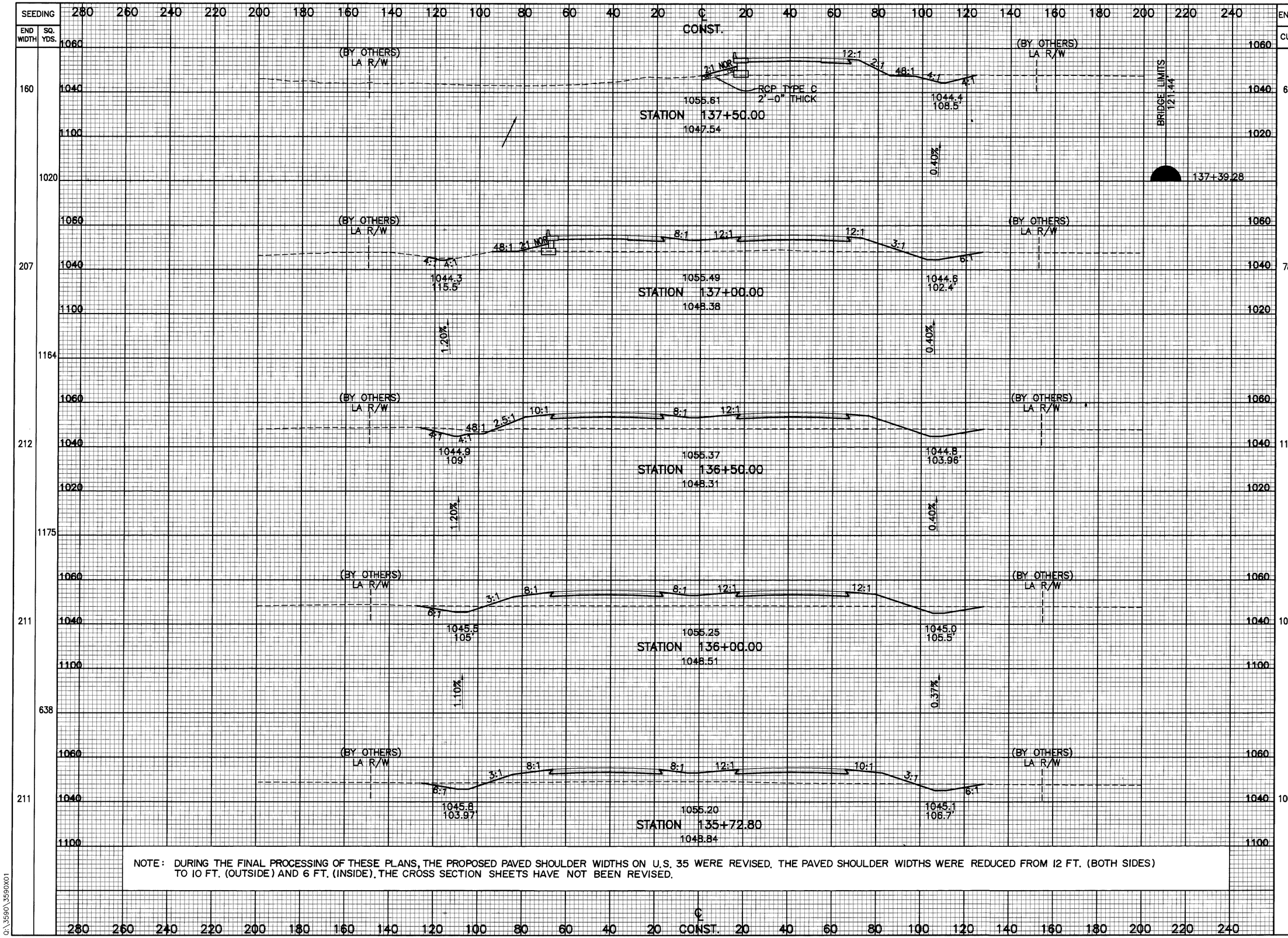
FAY-35-2.57

CROSS SECTIONS
STA. 135+72.80 TO STA. 137+50.00

REVISED 10-99 BY F Y C

CALCULATED J.L.G. CHECKED W.G.Y.

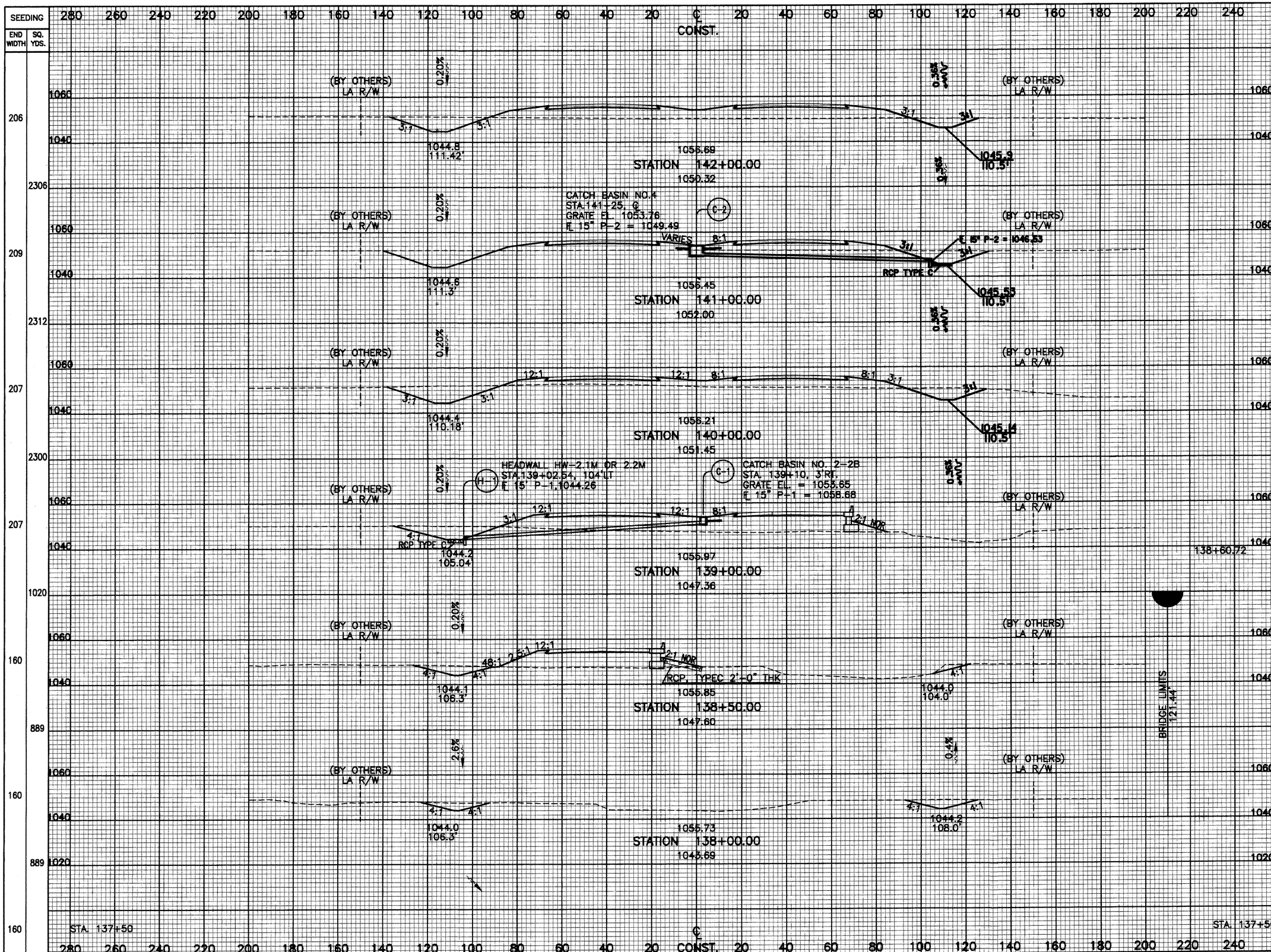
FAY-35-2.57



SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
160	1020	65	407	133	1146
207	1164	78	830	177	1610
212	1175	113	908	202	1669
211	638	105	894	104	870
211		100	814		

NOTE: DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE CROSS SECTION SHEETS HAVE NOT BEEN REVISED.

Q:\3590\3590X01



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
240	812		
		1060	2460
330	510		
		1220	1910
328	510		
		892	2913
154	1052		
		237	1421
102	482		
		212	447
126	0		
		177	377
65	407		

CALCULATED J.L.G.
CHECKED W.G.Y.

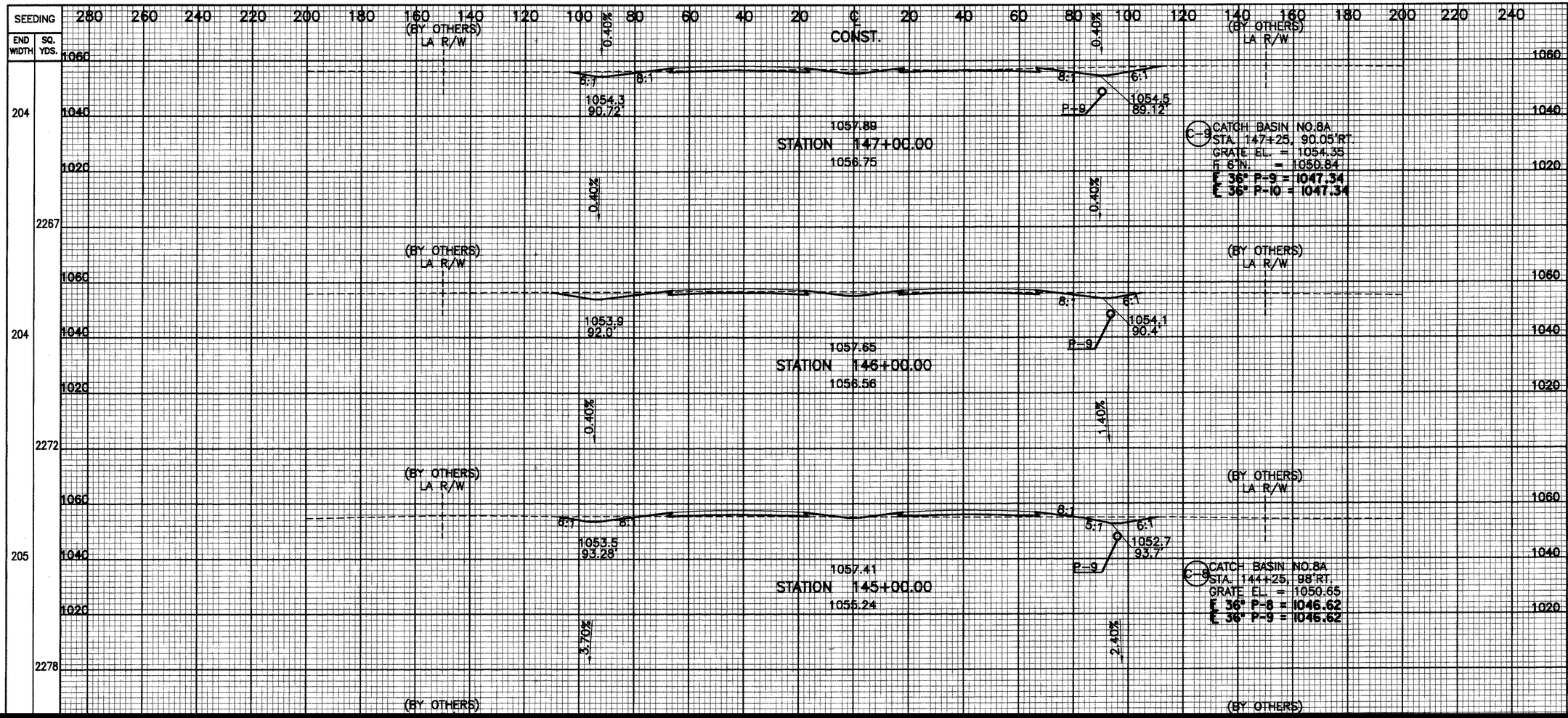
CROSS SECTIONS
STA. 138+50.00 TO STA. 142+00.00

FAY-35-2.57

REVISED 1-31-01 DSW

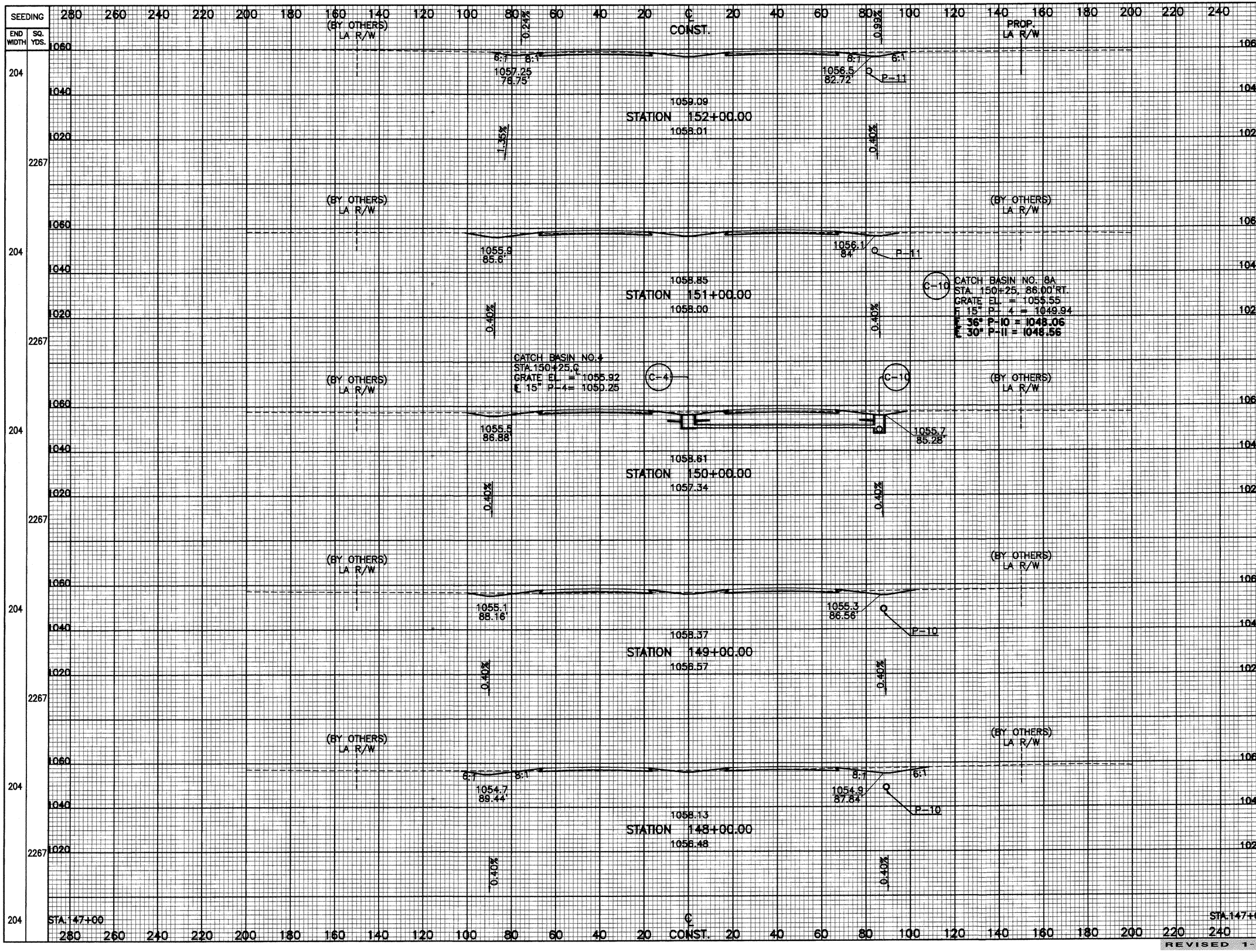
i:\users\dwise\fay\35\257&751\40lnewsht\new075.dgn 20-MAR-2001 3:38PM dwise
Q:\3590\3590X02

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
204	2267	149	8	561	32	
204	2272	154	9	413	202	
205	2278	69	100	220	1350	



CROSS SECTIONS

STA. 143+00.00 TO STA. 147+00.00



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
154	2		
		541	13
138	5		
		435	26
97	9		
		293	96
61	43		
		315	113
109	18		
		478	48
149	8		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 148+00.00 TO STA. 152+00.00

FAY-35-2.57

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&751\40lnewsht\new077.dgn 20-MAR-2001 3:40PM dwise
Q:\3590\3590X04

SEEDING END WIDTH	SEEDING SQ. YDS.	STA.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
206	2289	157+00.00	1933	0	6017	0
206	2283	156+00.00	1316	0	3800	0
205	2267	155+00.00	736	0	2062	0
204	2267	154+00.00	378	0	1181	0
204	2267	153+00.00	260	0	767	4
204		STA. 152+00	154	2		

CALCULATED J.L.G.
CHECKED W.G.Y.

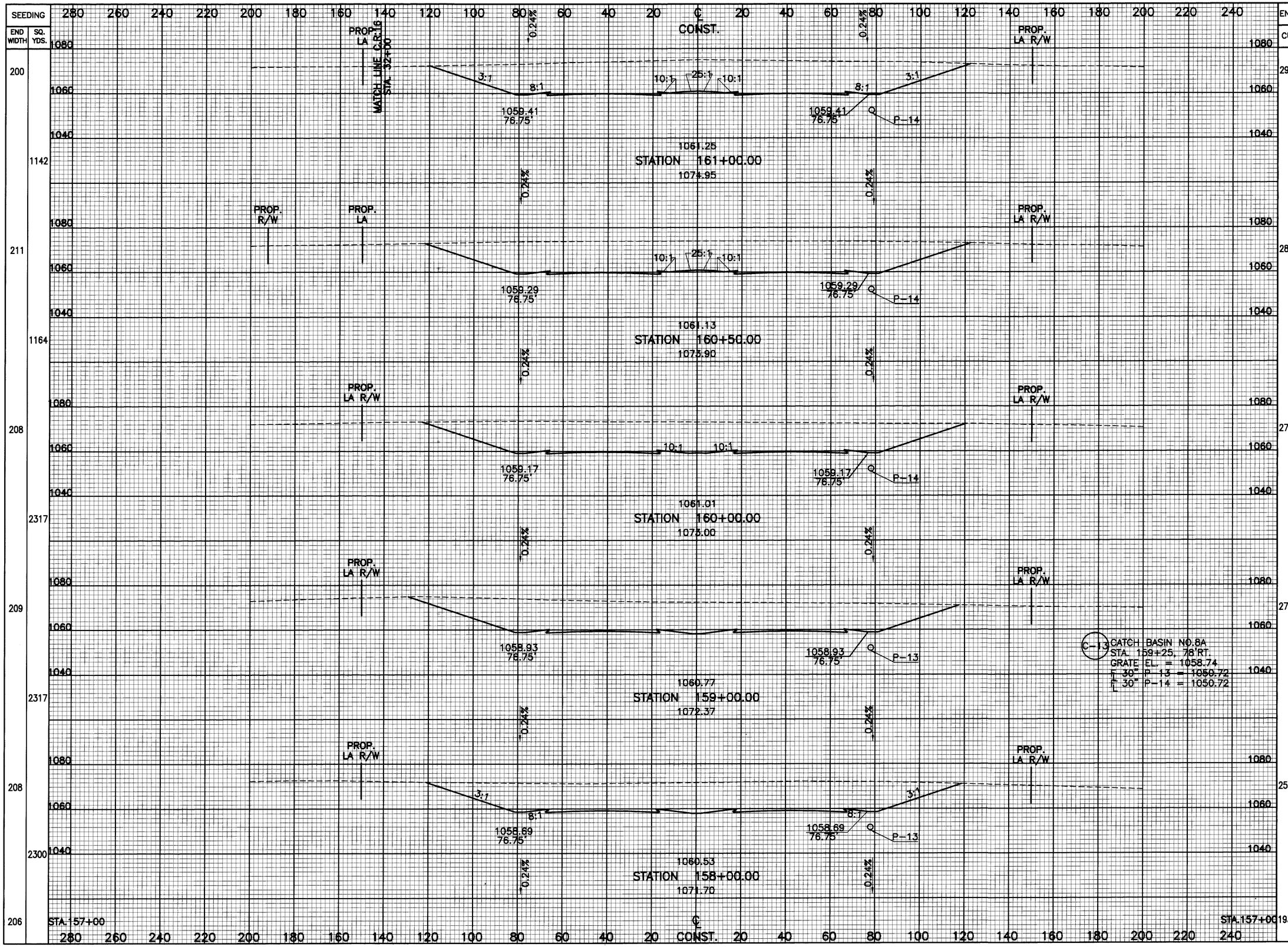
CROSS SECTIONS
STA. 153+00.00 TO STA. 157+00.00

FAY-35-2.57

78
522

Q:\3590\3590X05

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
200	1142	STATION 161+00.00	2902	0	5344	0
211	1164	STATION 160+50.00	2870	0	5191	0
208	2317	STATION 160+00.00	2736	0	10178	0
209	2317	STATION 159+00.00	2760	0	9902	0
208	2300	STATION 158+00.00	2587	0	8370	0
206		STA. 157+00	1933	0		



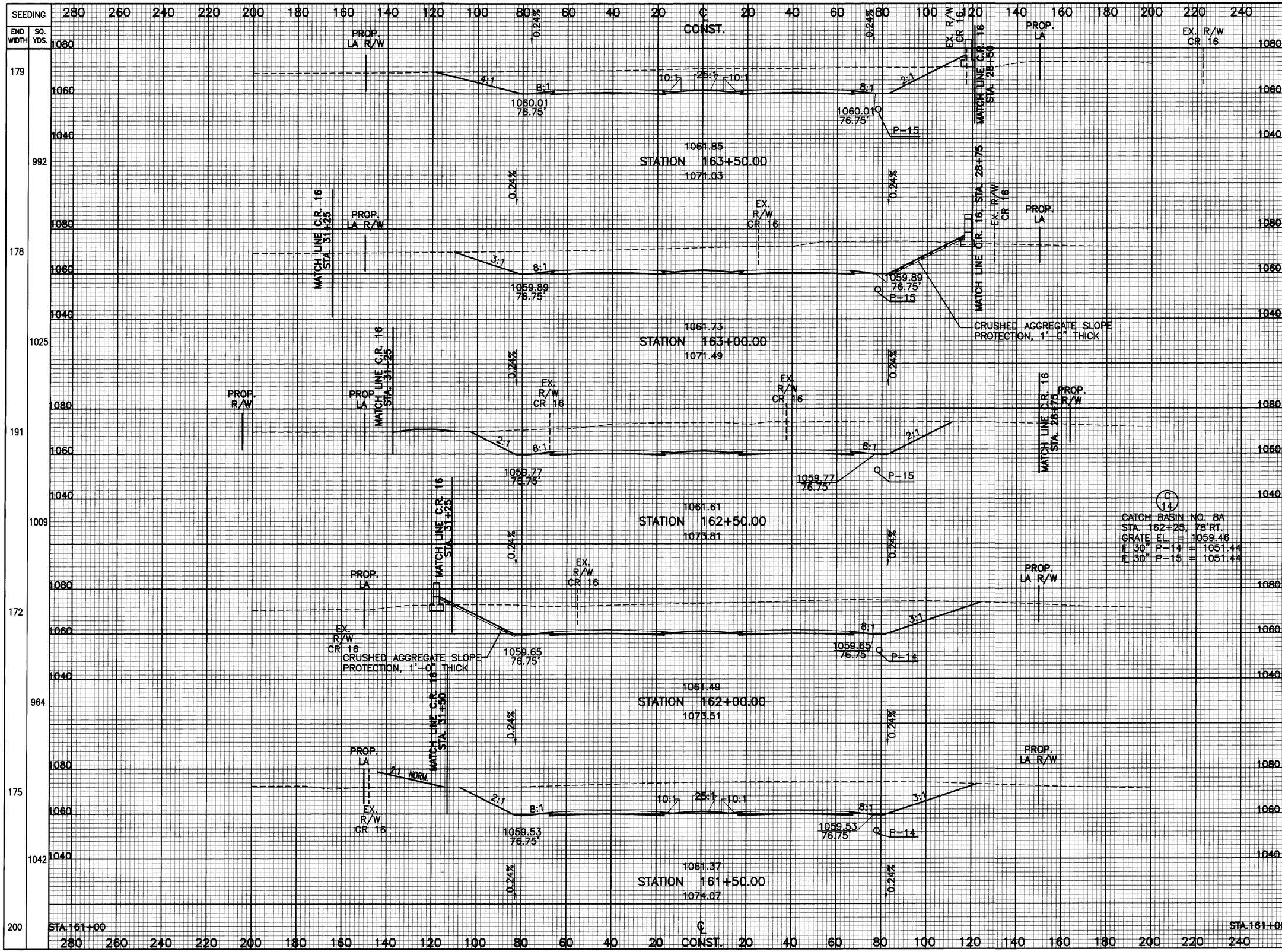
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 158+00.00 TO STA. 161+00.00

FAY-35-2.57

79
522

Q:\3590\3590X06



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
1998	47		
		3899	75
2213	34		
		4343	32
2477	0		
		4801	0
2708	0		
		5019	0
2713	0		
		5198	0
2902	0		

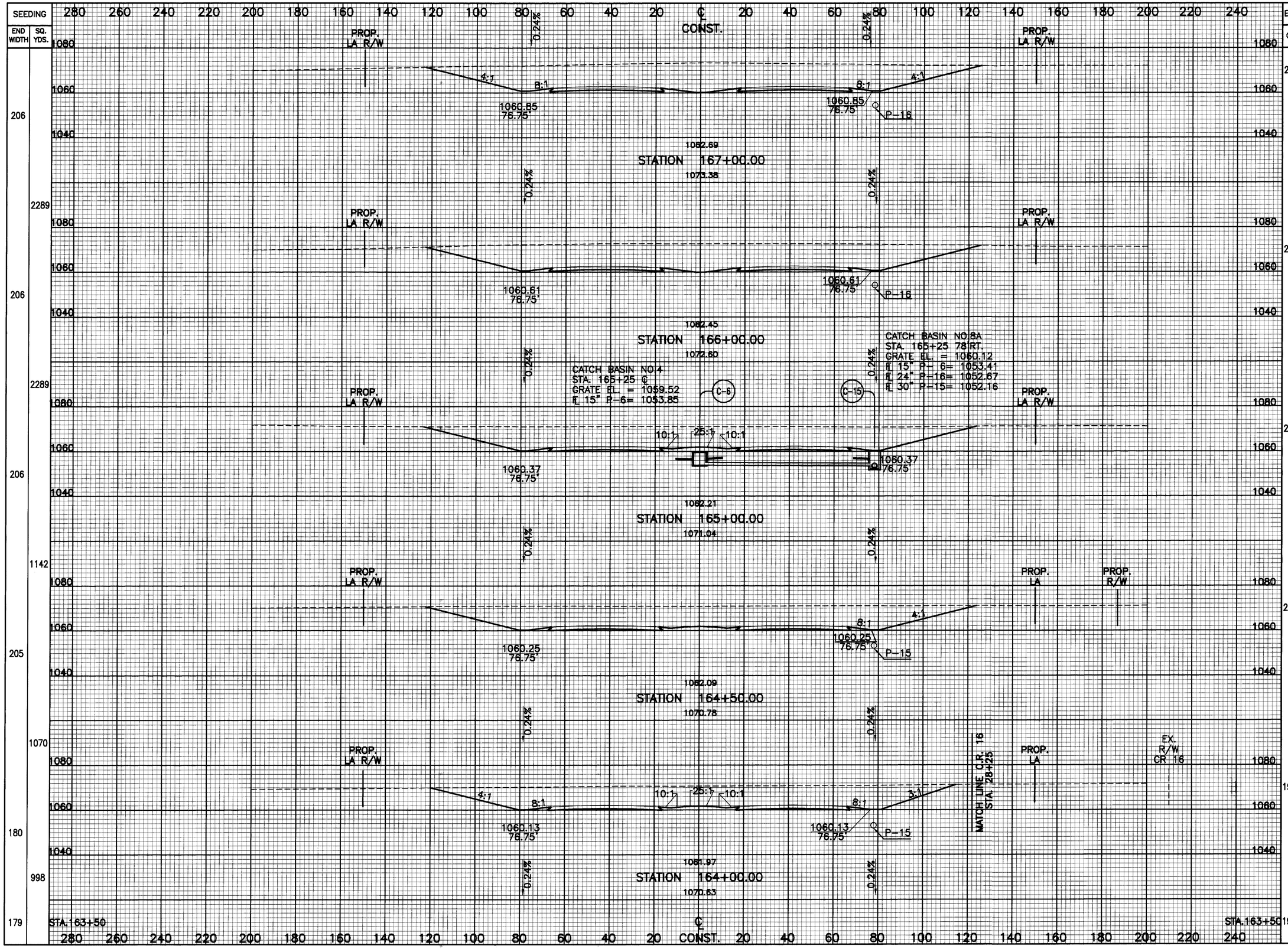
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 161+50.00 TO STA. 163+50.00

FAY-35-2.57

Q:\3590\3590X07

SEEDING		END AREA		VOLUME		CALCULATED
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	J.L.G.
						CHECKED
						W.G.Y.
206	2289	2396	0	8854	0	
206	2289	2385	0	8286	0	
206	2289	2089	0	3869	0	
205	1142	2089	0	3781	0	
180	1070	1994	0	3696	44	
179	998	1998	47			



CROSS SECTIONS

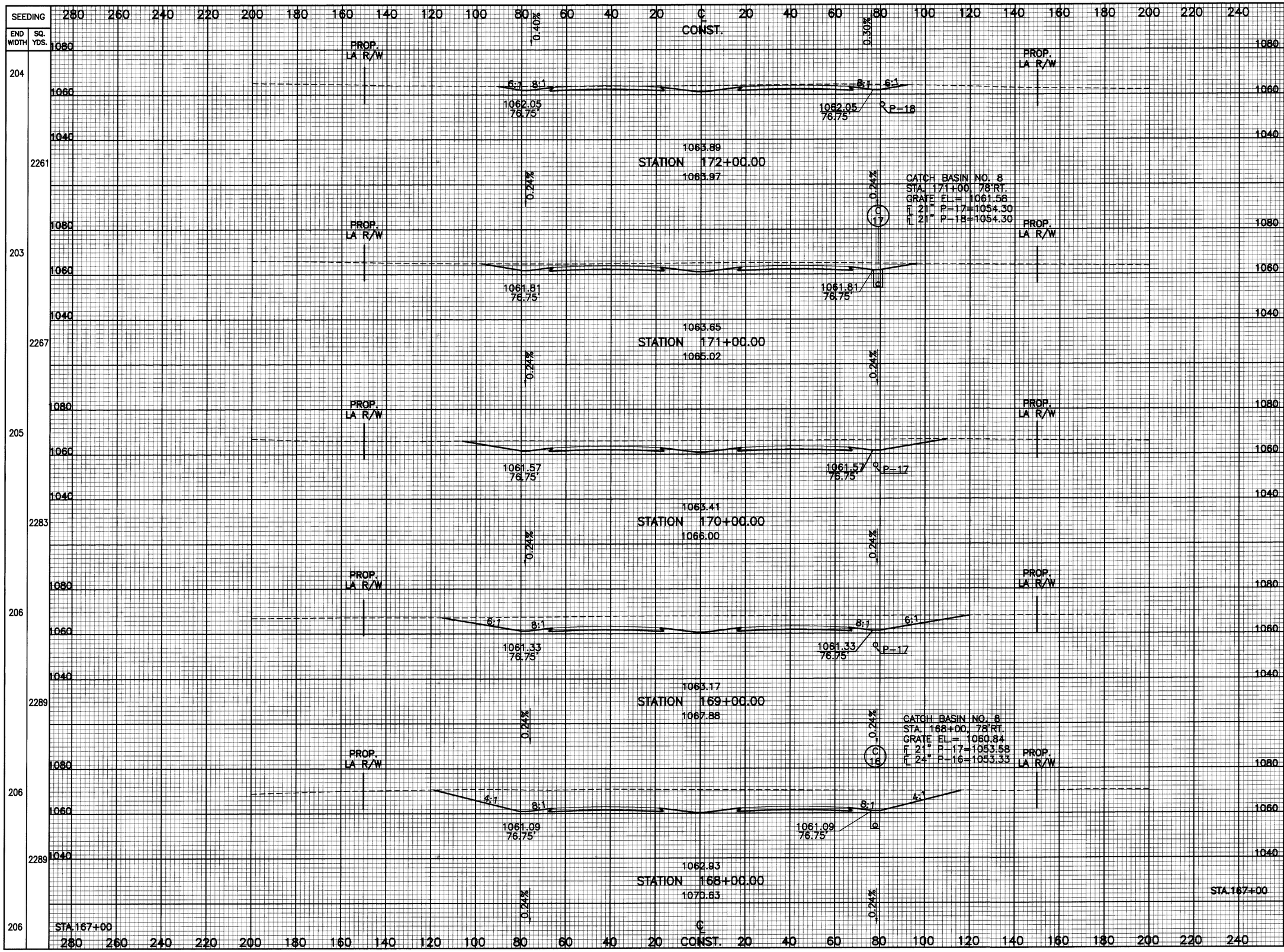
STA. 164+00.00 TO STA. 167+00.00

FAY-35-2.57

81
522

O:\3590\3590X8

SEEDING		END AREA		VOLUME		CALCULATED	CHECKED
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	J.L.G.	W.G.Y.
204	2261	301	0	1482	0		
203	2267	499	0	2421	0		
205	2283	808	0	3780	0		
206	2289	1233	0	5663	0		
206	2289	1825	0	7817	0		
206		2396	0				

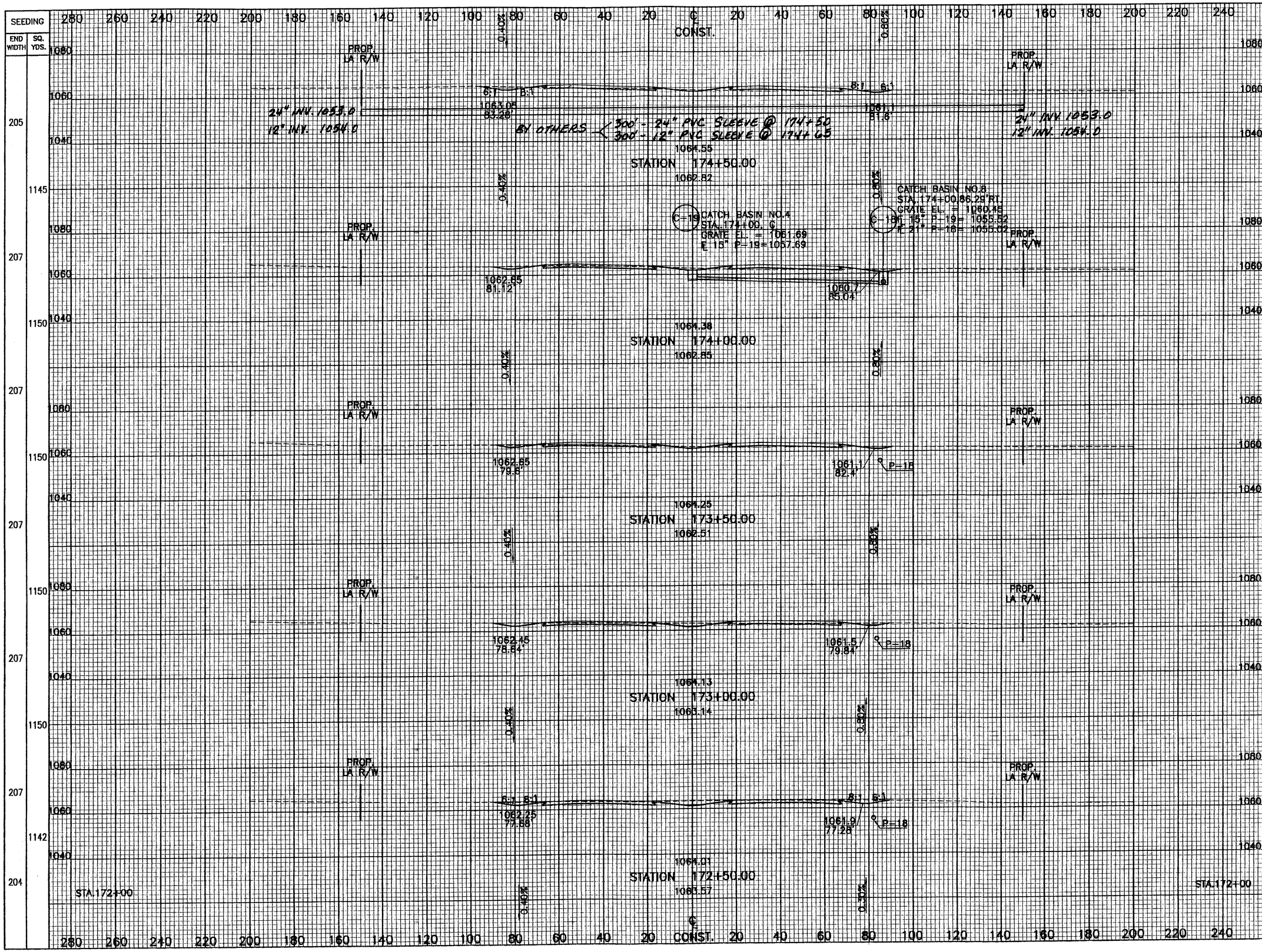


CROSS SECTIONS
STA. 168+00.00 TO STA. 172+00.00

FAY-35-2.57

82
522

Q:\3590\3590X09



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
57	43		
		130	75
83	38		
		123	69
49	36		
		153	38
116	5		
		297	5
204	0		
		418	0
301	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 172+50.00 TO STA. 174+50.00

FAY-35-2.57

Q:\3590\3590X10

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
207		177+00.00	42	157		
	1150				107	226
207		176+50.00	73	87		
	1148				138	121
206		176+00.00	75	43		
	1145				221	54
206		175+50.00	163	15		
	1145				300	26
206		175+00.00	160	13		
	1142				201	52
205		174+50	57	43		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 175+00.00 TO STA. 177+00.00

FAY-35-2.57

84
522

Q:\3590\3590X11

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
209	1159	179+50.00	33	897	56	1530
208	1150	179+00.00	27	755	50	1275
206	1145	178+50.00	26	622	56	983
206	1148	178+00.00	34	439	66	644
207		177+50.00	37	256	74	383
			42	157		

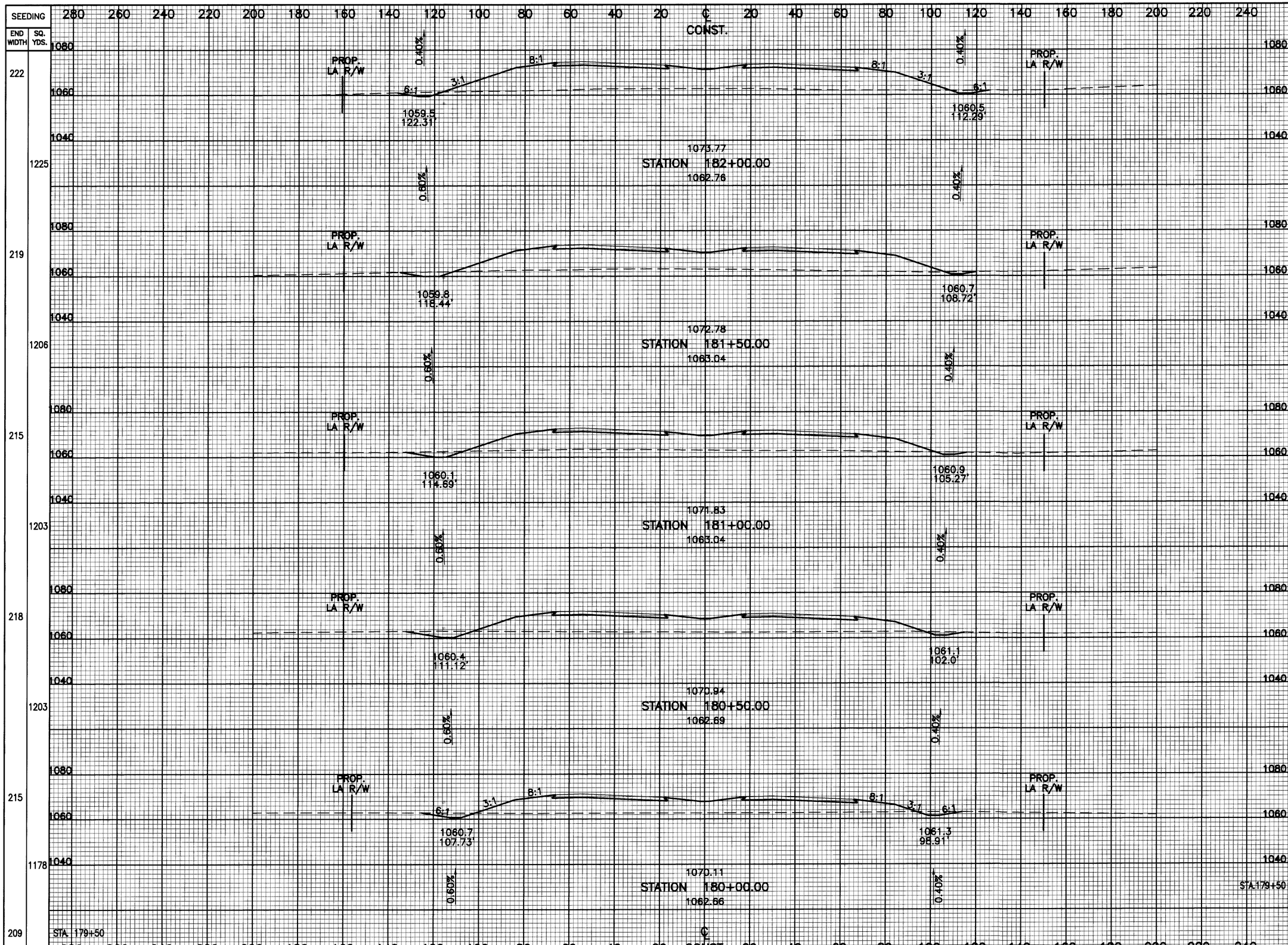
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 177+50.00 TO STA. 179+50.00

FAY-35-2.57

85
522

Q:\3590\3590X12



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
36	1902		
		69	3262
38	1621		
		79	2755
47	1354		
		103	2361
64	1195		
		106	2082
50	1053		
		77	1806
33	897		

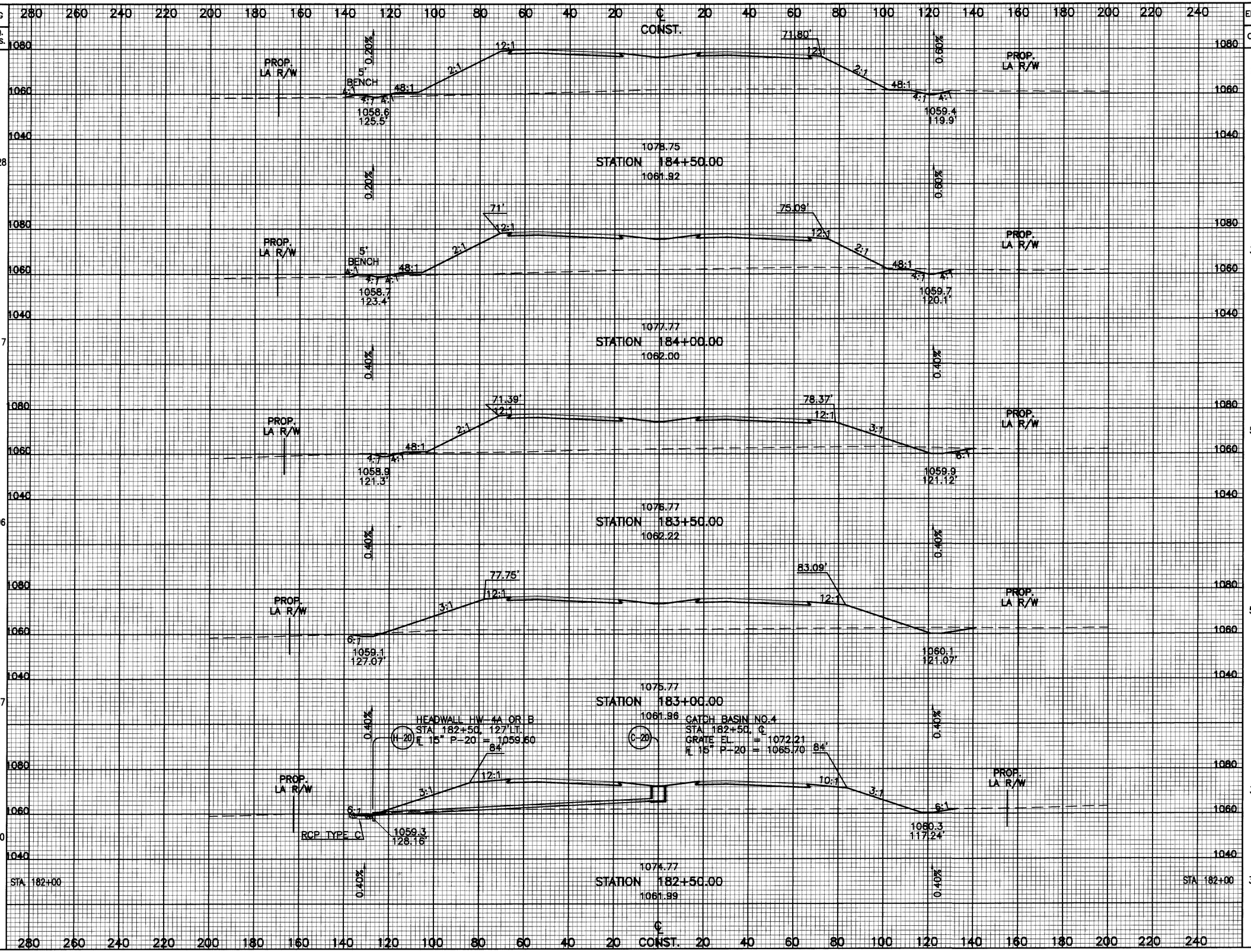
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 180+00.00 TO STA. 182+00.00

FAY-35-2.57

Q:\3590\3590X13

SEEDING END WIDTH	SEEDING SQ. YDS.
239	1328
239	1317
235	1306
235	1287
228	1250
222	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
21	2882		
		51	5088
34	2613		
		85	4658
57	2417		
		102	4463
53	2402		
		78	4327
31	2271		
		63	3864
36	1902		

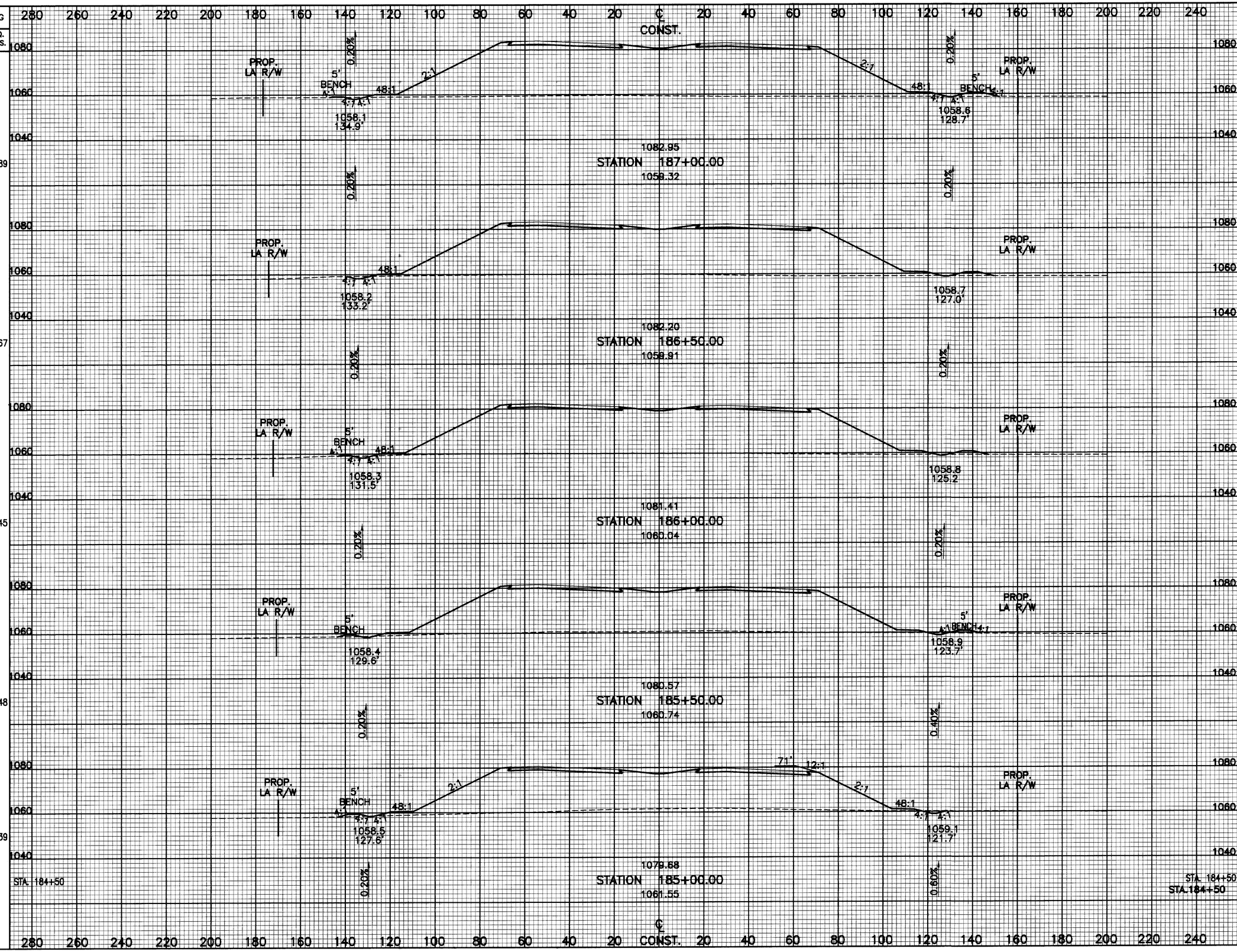
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 182+50.00 TO STA. 184+50.00

FAY-35-2.57

87
522

Q:\3590\3590X14



SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
250		6	4260		
	1389			13	7637
250		7	3987		
	1367			14	7159
242		8	3744		
	1345			14	6660
242		7	3448		
	1348			15	6130
243		9	3172		
	1339			28	5606
239		21	2882		

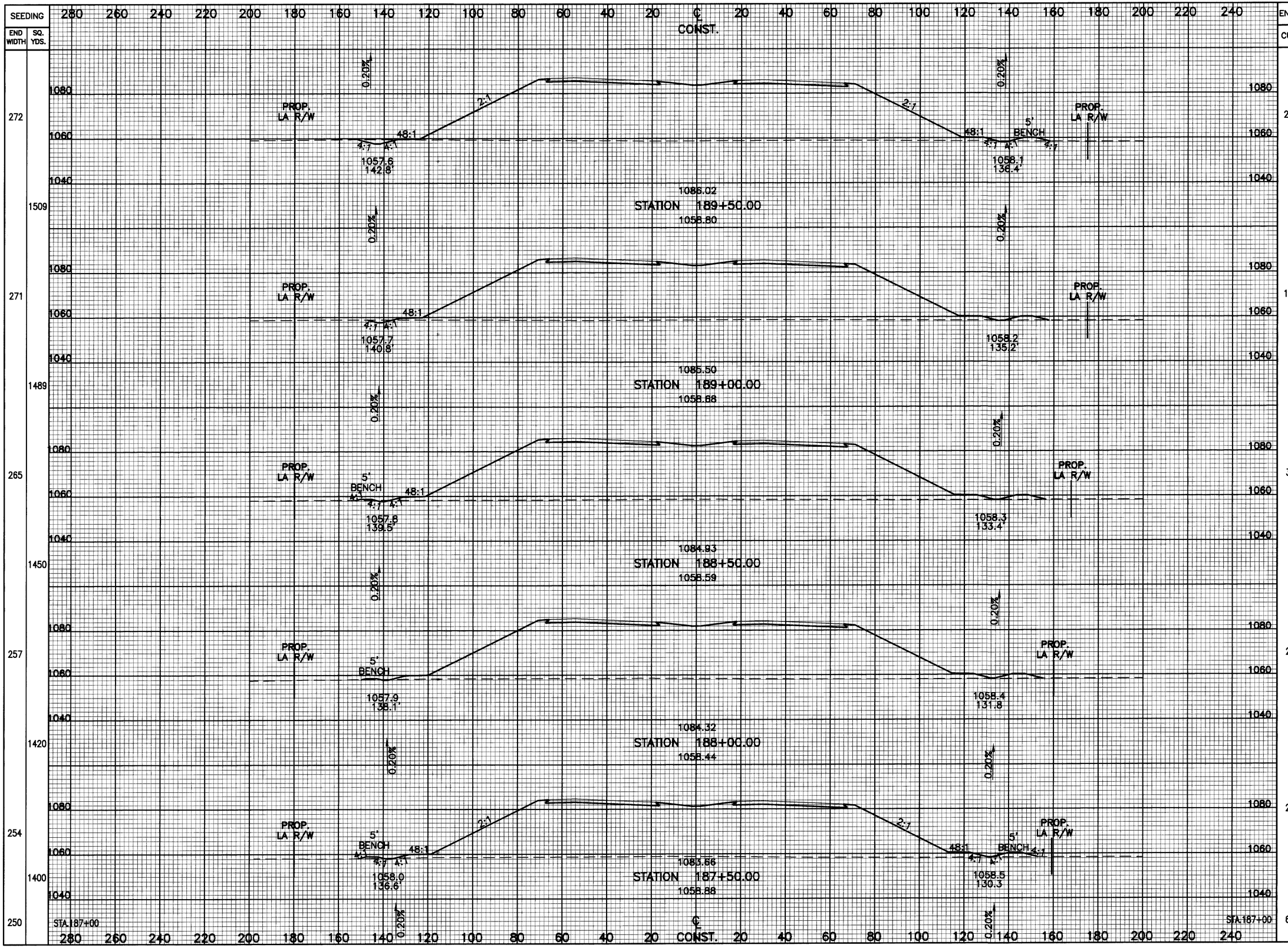
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 185+00.00 TO STA. 187+00.00

FAY-35-2.57

Q:\3590\3590X15

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
272	1509	20	5021	37	9264	
271	1489	19	4984	21	9170	
265	1450	3	4919	5	8987	
257	1420	2	4786	4	8649	
254	1400	2	4554	8	8162	
250		6	4260			



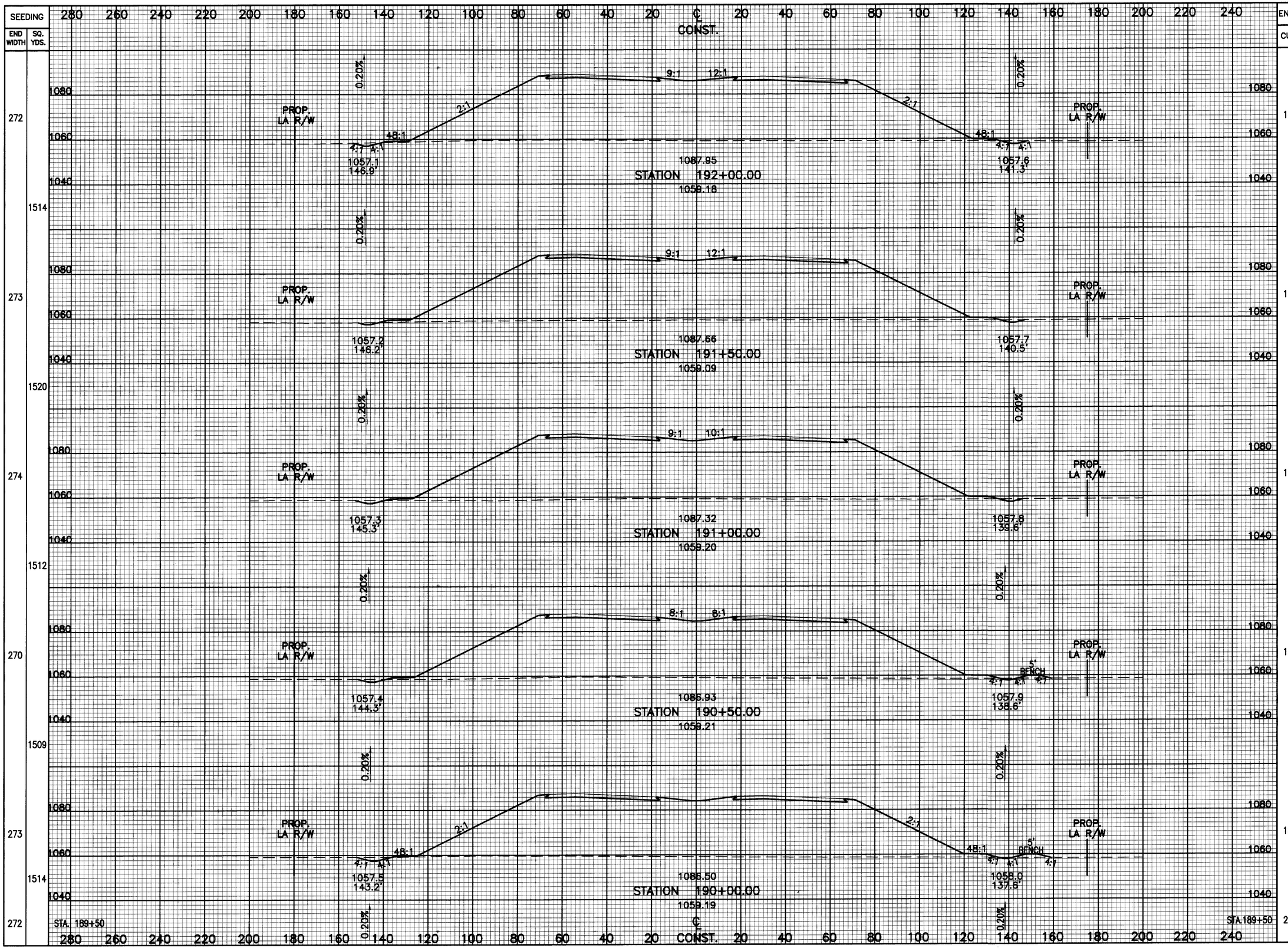
CROSS SECTIONS
STA. 187+50.00 TO STA. 189+50.00

FAY-35-2.57

89
522

Q:\3590\3590X16

SEEDING END WIDTH	SEEDING SQ. YDS.
272	1514
273	1520
274	1512
270	1509
273	1514
272	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
18	5433		
		28	10082
12	5455		
		27	10016
17	5362		
		28	9814
13	5237		
		29	9553
18	5080		
		36	9353
20	5021		

CALCULATED J.L.G.
CHECKED W.G.Y.

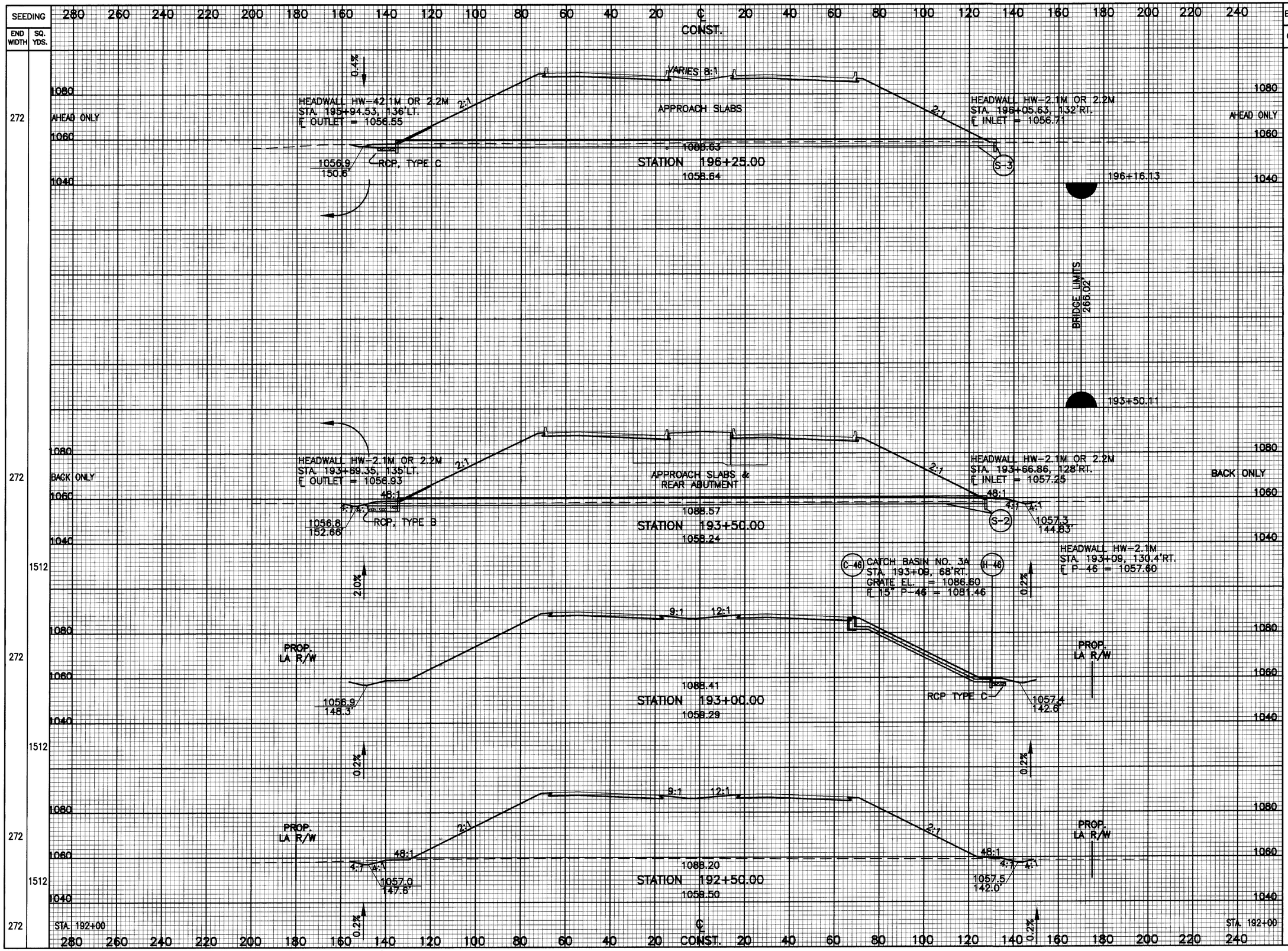
CROSS SECTIONS
STA. 190+00.00 TO STA. 192+00.00

FAY-35-2.57

Q:\3590\3590X17

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
272	AHEAD ONLY	7	5964		
272	BACK ONLY	14	6054		
1512		39	10717		
272		28	5520		
1512		55	10153		
272		31	5445		
1512		46	10073		
272		18	5433		

CALCULATED J.L.G.
CHECKED W.G.Y.



CROSS SECTIONS
STA. 192+50.00 TO STA. 196+25.00

FAY-35-2.57

Q:\3590\3590X18

SEEDING	
END WIDTH	SQ. YDS.
255	1420
256	1423
256	1425
257	1417
253	730
272	

END AREA		VOLUME		CALCULATED J.L.G. / CHECKED W.G.Y.
CUT	FILL	CUT	FILL	
24	5025			
		55	9206	
35	4917			
		78	8902	
49	4697			
		120	8757	
80	4760			
		116	9335	
45	5321			
		24	5225	
7	5964			

CROSS SECTIONS
STA. 196+50.00 TO STA. 198+50.00

FAY-35-2.57

92
522

Q:\3590\3590X19

SEEDING END WIDTH	SEEDING SQ. YDS.
257	1420
254	1409
253	1409
254	1412
254	1414
255	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

CATCH BASIN NO.3A
STA.200+90.58'RT.
GRATE EL. =1083.76
C-48 L 15° P-48=1078.62 H-48

HEAD WALL HW-2.1M
STA.200+90.125.04'RT.
R 15° P-48=1057.42

PROP. LA R/W

0.40%

2:1

4:1

4:1 48:1

1056.9
144.2

1086.50
STATION 201+00.00
1059.61

RCP TYPE C

2.60%

48:1

4:1

1058.3
133.0'

0.4%

1086.02
STATION 200+50.00
1060.22

1056.2
144.9

1058.5
123.3'

4.4%

1086.50
STATION 200+00.00
1060.41

1058.40
139.4'

1058.7
123.9'

0.4%

1086.93
STATION 199+50.00
1060.49

5' BENCH

1058.2
140.7'

1059.1
124.7'

VAR. 8:1

1087.32
STATION 199+00.00
1060.34

1058.0
141.9'

1059
124.7'

0.60%

STA.198+50

STA.198+50

℄ CONST.

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
43	4714		
		57	8834
18	4826		
		38	8922
23	4809		
		44	3998
24	4908		
		56	9200
36	5028		
		56	9309
24	5025		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 199+00.00 TO STA. 201+00.00

FAY-35-2.57

93
522

Q:\3590\3590X20

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
258	1428	48	4371	98	8073	
256	1425	57	4347	120	8009	
257	1434	72	4302	145	7897	
259	1445	84	4226	152	7999	
261	1439	80	4412	114	8450	
257		43	4714			

CROSS SECTIONS
STA. 201+50.00 TO STA. 203+50.00

FAY-35-2.57

94
522

STATION 203+50.00
1082.20
1057.76
1055.1
139.24'
1055.0
135.0'

STATION 203+00.00
1082.95
1059.05
1055.2
140.7'
1055.5
133.5'

STATION 202+50.00
1083.66
1060.48
1055.4
141.8'
1056.0
133.9'

STATION 202+00.00
1084.32
1063.55
1055.6
142.7'
1056.5
134.2'

STATION 201+50.00
1084.93
1061.80
1055.8
143.5'
1057.0
134.4'

PROP. LA R/W

CONST.

Q:\3590\3590X21

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
262			6	3963		
	1459	206+00.00			11	7429
263			5	4060		
	1462	205+50.00			20	7613
263			16	4162		
	1450	205+00.00			27	7865
259			13	4332		
	1434	204+50.00			36	8049
257			25	4361		
	1431	204+00.00			68	8086
258	STA. 203+50		48	4371		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 204+00.00 TO STA. 206+00.00

FAY-35-2.57

95
522

Q:\3590\3590X22

SEEDING END WIDTH	SEEDING SQ. YDS.
260	1448
261	1462
265	1467
263	1467
265	1464
262	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
2	3039		
		3	5833
1	3260		
		2	6168
1	3401		
		2	6512
1	3631		
		3	6899
2	3820		
		8	7207
6	3963		

CALCULATED J.L.G.
CHECKED W.G.Y.

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 / 1060 / 1040

PROP. LA R/W

5' BENCH EL. 1057.0

4:1 4:1 4:1 4:1

2:1

0.10%

1054.70 122.70'

1075.43
STATION 208+50.00
1054.99

1054.65 124.32'

1074.19
STATION 208+00.00
1054.65

1054.60 126.06'

1075.01
℄ STATION 207+50.00
1054.59

1054.55 127.90'

1075.88
STATION 207+00.00
1054.66

HEADWALL HW2.1M OR 2.2M
STA. 206+50, 114.5' LT.
℄ OUTLET=1054.50

RCP TYPE C

0.20%

1054.50 129.84'

1076.80
STATION 206+50.00
1054.91

HEAD WALL HW 2.1M OR 2.2M
STA. 206+50, 109.5'RT.
℄ INLET = 1054.59

S-4

EX. C.B.
STA. 206+49, 167.67'RT.
GRATE EL. = 1056.86
℄ 18" = 1050.19

STA. 206+00

CROSS SECTIONS
STA. 206+50.00 TO STA. 208+50.00

FAY-35-2.57

96
522

Q:\3590\3590X23

SEEDING END WIDTH	SEEDING SQ. YDS.
260	1442
259	1445
261	1456
263	1459
262	1450
260	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 $\mathbb{C}_L$ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 / 1060 / 1040

PROP. LA R/W

5' BENCH EL. 1057.0

0.10%

0.20%

0.1%

4:1 4:1 4:1 2:1 3:1 6:1

1054.95 122.17'

1055.1 118.40'

1070.40 STATION 211+00.00 1055.71

1054.90 120.45'

1055.0 121.53'

1070.90 STATION 210+50.00 1055.67

HEADWALL HW=2.1M OR 2.2M STA. 210+00, 104'LT. ℄ OUTLET = 1054.62

HEADWALL HW=2.1M OR 2.2M STA. 210+00, 119'RT. ℄ OUTLET = 1054.71

1054.85 118.87'

RCP TYPE C

S-4A

1054.85 121.25'

1071.45 STATION 210+00.00 1055.59

1054.80 119.75'

1072.06 STATION 209+50.00 1055.49

HEADWALL HW=2.1M STA. 208+98, 107.21'LT. ℄ 15" P-21 = 1054.90

(H-21)

CATCH BASIN NO.4 STA. 208+98, ℄ GRATE EL. = 1070.16 ℄ 15" P-21= 1064.87

(C-21)

1054.75 121.18'

1072.72 STATION 209+00.00 1056.35

STA. 208+50

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
18	2501		
		36	4734
20	2611		
		28	4861
10	2638		
		19	4957
10	2715		
		14	5125
5	2820		
		7	5425
2	3039		

CALCULATED J.L.G.
CHECKED W.G.Y.

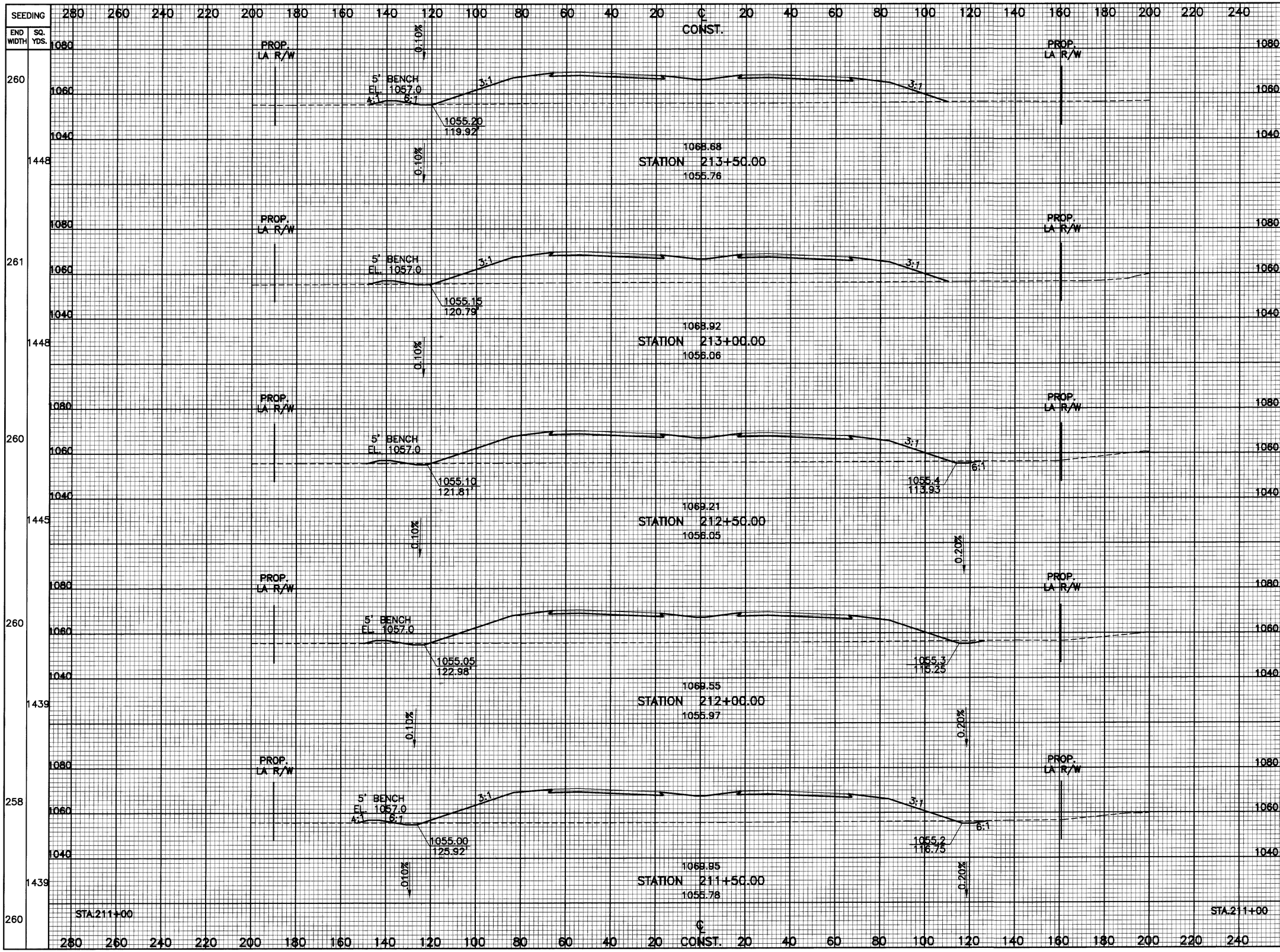
CROSS SECTIONS

STA. 209+00.00 TO STA. 211+00.00

FAY-35-2.57

97
522

Q:\3590\3590X24



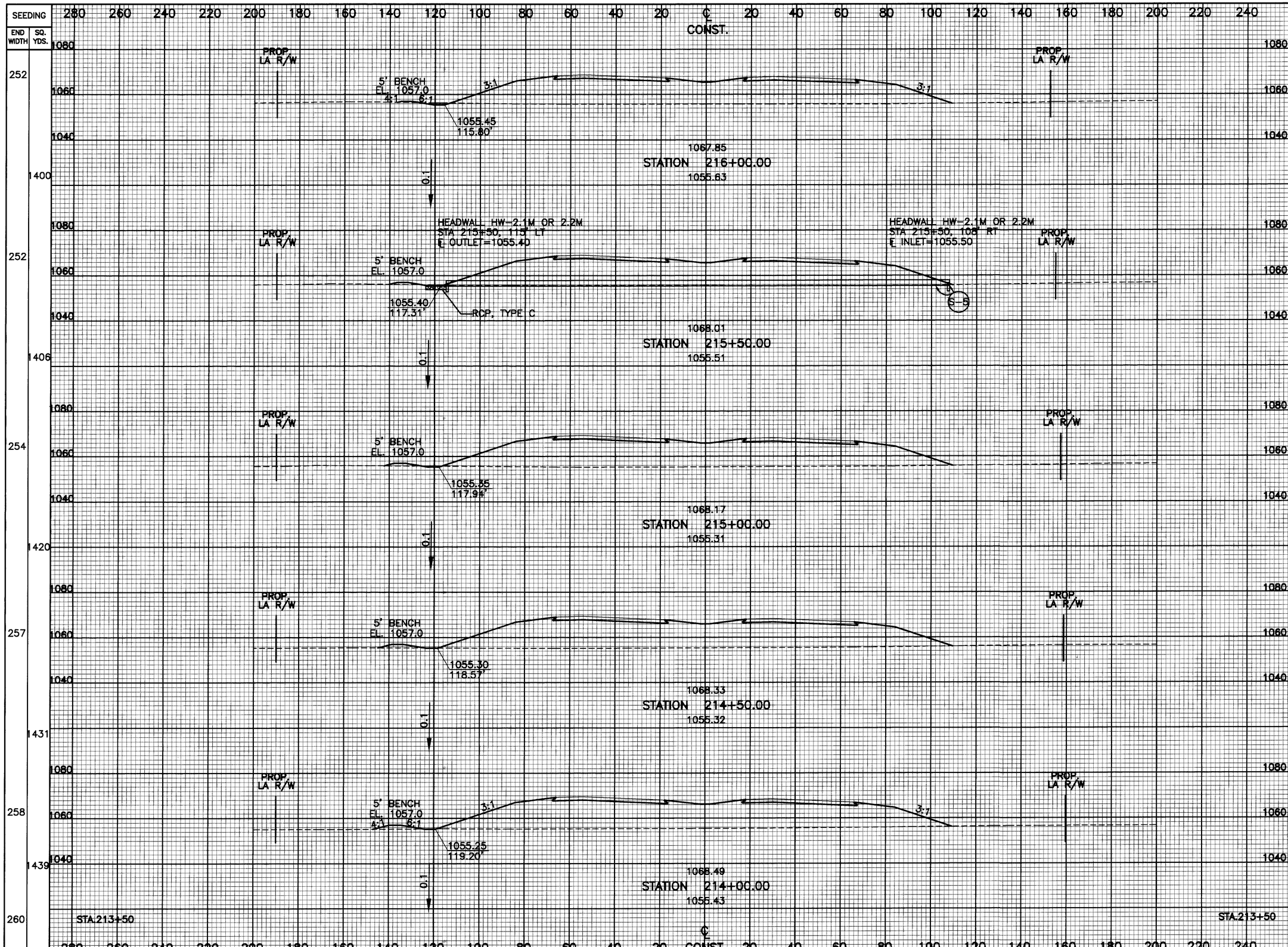
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
2	2221		
		7	4108
5	2215		
		23	4184
19	2303		
		38	4324
21	2366		
		38	4537
19	2534		
		35	4662
18	2501		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 211+50.00 TO STA. 213+50.00

FAY-35-2.57

Q:\3590\3590X25



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
11	2038		
		15	3858
5	2128		
		11	4000
6	2192		
		9	4076
3	2210		
		5	4272
2	2403		
		4	4282
2	2221		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 214+00.00 TO STA. 216+00.00

FAY-35-2.57

Q:\3590\3590X26

SEEDING END WIDTH	SEEDING SQ. YDS.
220	2528
235	1331
244	1367
248	1381
249	1392
252	

HEADWALL HW-2.1M OR 2.2M
STA 219+00, 107' LT
PROP. ℄ 15" P-22=1055.75

CATCH BASIN NO 4
STA 219+00 ℄
GRATE EL=1064.14
℄ 15" P-22=1060.10

STATION 219+00.00
1066.89
1056.58
1055.75 / 109.17'
1056.65 / 106.47'

STATION 218+00.00
1067.21
1057.38
1055.65 / 110.58'

STATION 217+50.00
1067.37
1056.47
1055.60 / 111.81'

STATION 217+00.00
1067.53
1055.81
1055.55 / 113.07'

STATION 216+50.00
1067.69
1056.53
1055.50 / 114.30'

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
12	1581		
		39	5867
9	1587		
		14	3065
6	1723		
		11	3362
5	1907		
		12	3610
7	1991		
		17	3731
11	2038		

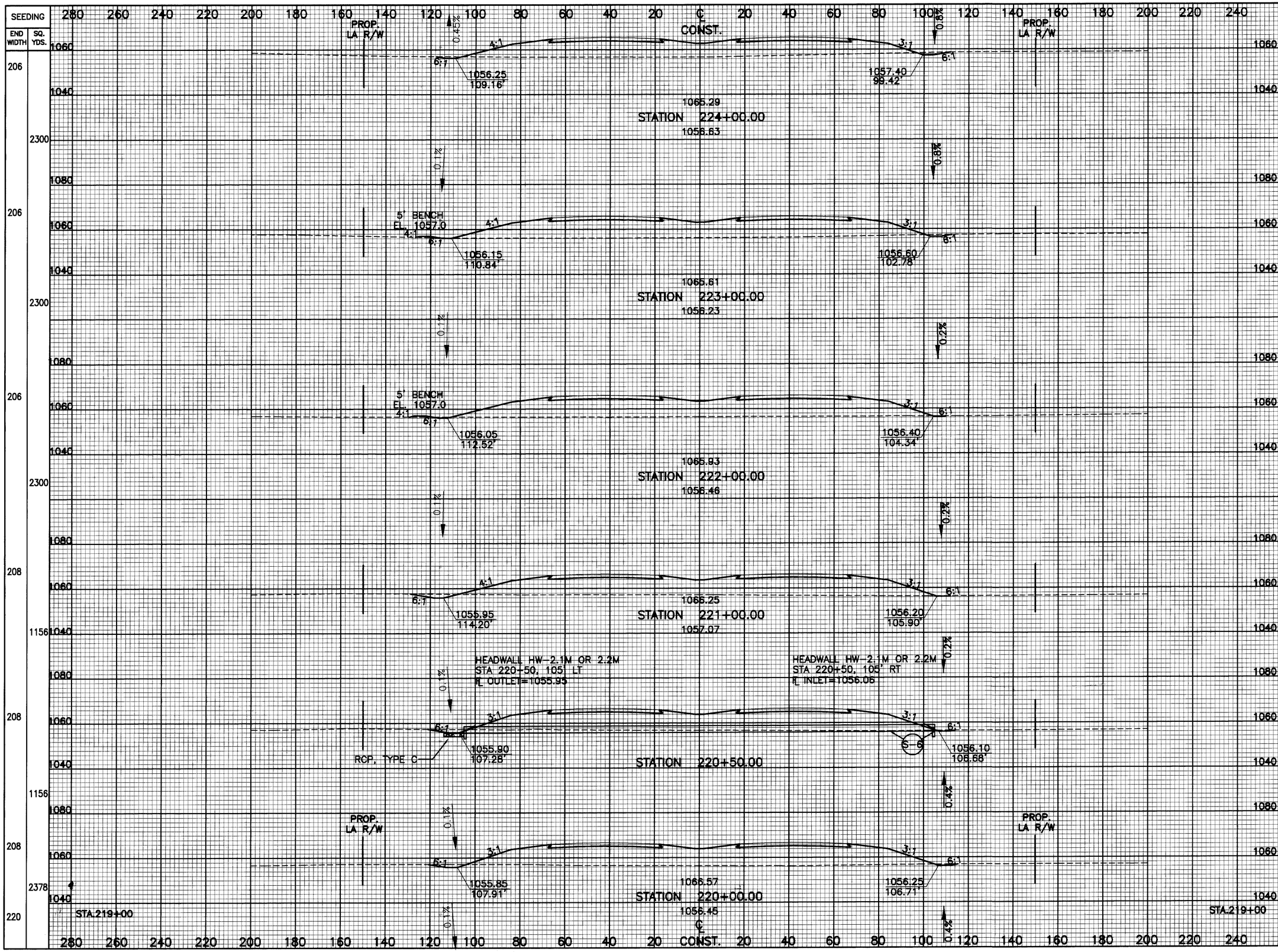
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 216+50.00 TO STA. 219+00.00

FAY-35-2.57

100
522

Q:\3590\3590X27



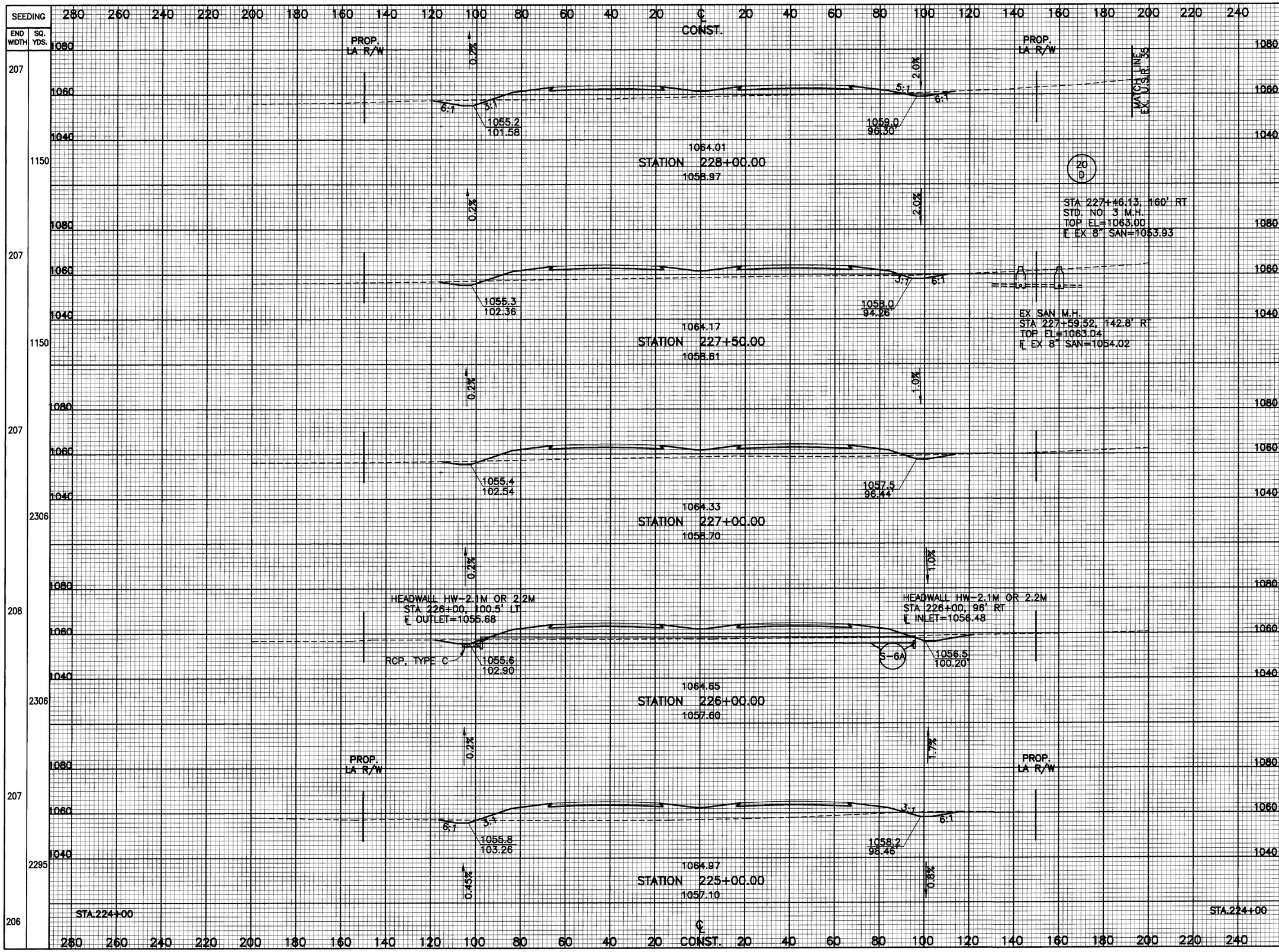
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
25	1201		
		69	4813
12	1398		
		38	5276
8	1451		
		60	5260
24	1389		
		44	2621
23	1441		
		43	2773
23	1553		
		65	5804
12	1581		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 220+00.00 TO STA. 224+00.00

FAY-35-2.57

Q:\3590\3590X28



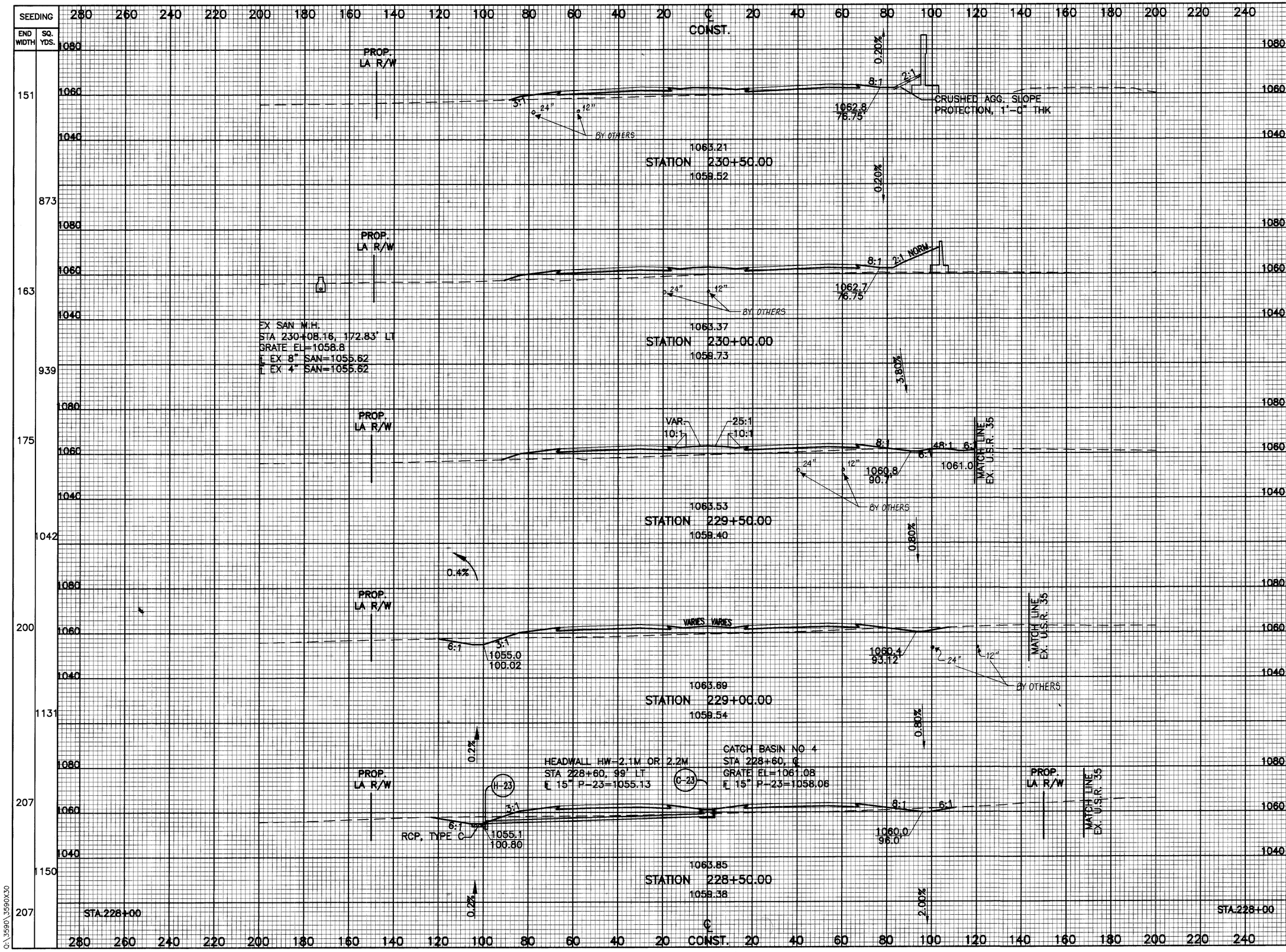
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
63	584		
		102	1197
47	708		
		86	1346
45	745		
		212	3062
69	908		
		213	3560
46	1014		
		132	4102
25	1201		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 225+00.00 TO STA. 228+00.00

FAY-35-2.57

Q:\3590\3590X29



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
0	495		
		0	1015
0	601		
		24	951
25	426		
		100	781
82	417		
		155	813
85	460		
		137	967
63	584		

REVISED 1/00 BY FYC

CALCULATED J.L.G.

CHECKED W.G.Y.

CROSS SECTIONS
STA. 228+50.00 TO STA. 230+50.00

FAY-35-2.57

Q:\3590\3590X30

SEEDING END WIDTH	SEEDING SQ. YDS.
153	878
163	942
176 / 282	1487
253 / 162	862
148	831
151	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
4	453		
		83	977
85	602		
		106	1236
AHEAD 29 / BACK 31	732 / 1822		
		44	2868
AHEAD 16 / BACK 16	1275 / 467		
		15	788
0	384		
		0	814
0	495		

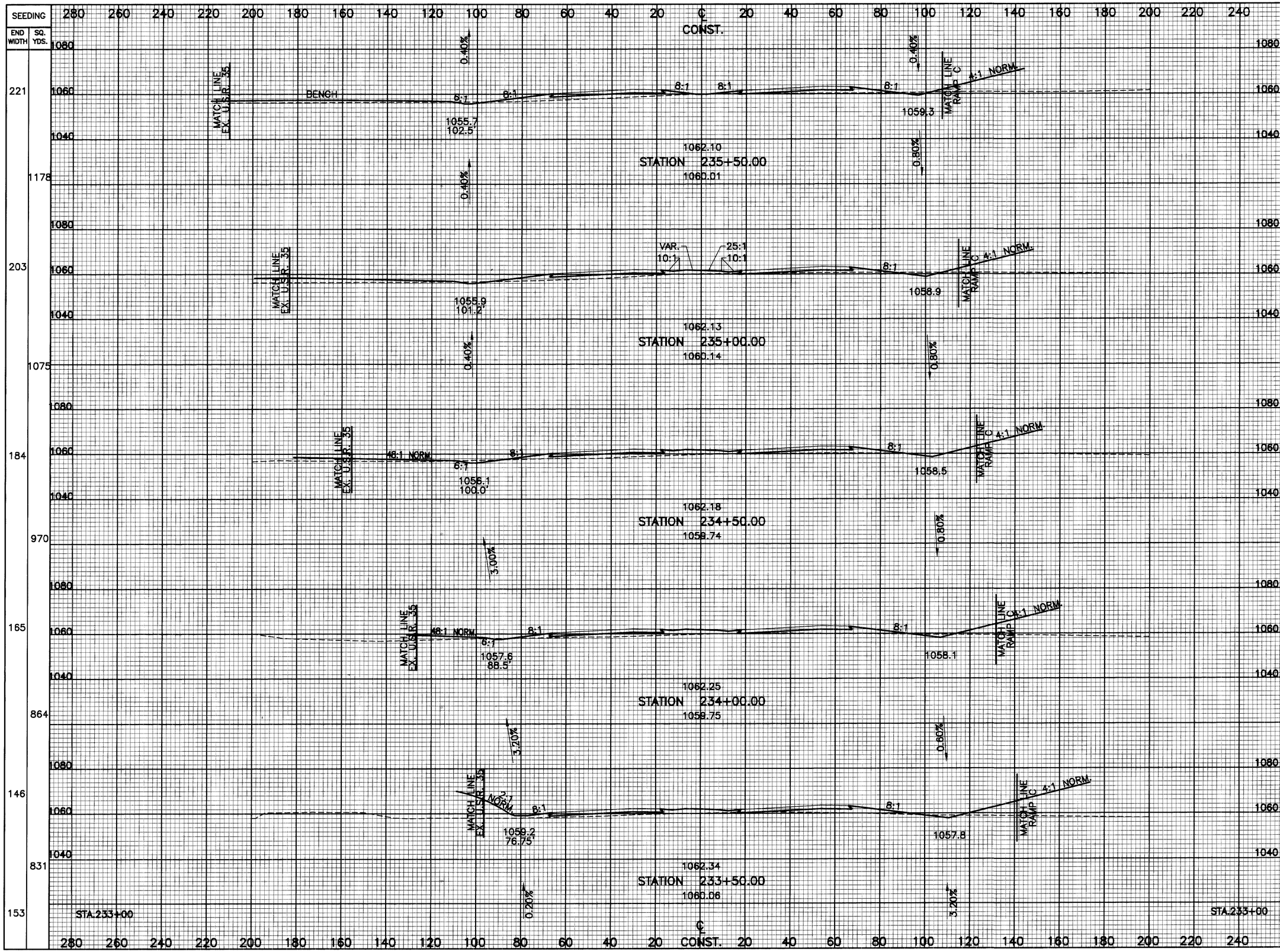
CALCULATED J.L.G.
CHECKED W.G.Y.
REVISED 1/00 BY FYC

CROSS SECTIONS
STA. 231+00.00 TO STA. 233+00.00

FAY-35-2.57

104
522

Q:\3590\3590X31



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
22	331		
		45	632
26	351		
		49	622
26	320		
		43	635
20	365		
		31	699
13	389		
		16	780
4	453		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 233+50.00 TO STA. 235+50.00

FAY-35-2.57

Q:\3590\3590X32

SEEDING END WIDTH	SEEDING SQ. YDS.
210	1262
244	1453
279	1495
259	1395
243	1289
221	

CONST. ℄ CONST.

MATCH LINE RAMP D

MATCH LINE RAMP C

0.20%

8:1

6:1

4:1 NORM.

1055.9
102.88'

1062.25
STATION 238+00.00
1061.66

0.82%

1055.8
102.32'

1062.18
STATION 237+50.00
1060.00

0.60%

1055.7
102.72'

1062.13
STATION 237+00.00
1059.17

0.46%

MATCH LINE EX. U.S.R. 35

6:1 NORM. 6:1 NORM.

1055.1
219.31'

1055.6
103.28'

1059.7

1062.10
STATION 236+50.00
1058.73

0.40%

6' 2' 6'

4:1 6:1 6:1 EX. GRND.

OUTLET DITCH

H-25 HEADWALL HW-4A OR B
STA 236+00, 90° LT.
℄ 18" P=25=1055.55

C-25 CATCH BASIN NO 4
STA 236+00, 90° RT.
GRATE EL=1059.61
℄ 18" P=25=1056.13

0.28%

5:1 NORM.

OUTLET DITCH

1055.5
104.1'

RCP, TYPE C

1059.5

1062.09
STATION 236+00.00
1058.03

STA.235+50

STA.235+50

℄

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
202	8	225	151
40	155	55	391
19	267	49	550
33	327	51	637
22	360	41	640
22	331		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 236+00.00 TO STA. 238+00.00

FAY-35-2.57

106
522

Q:\3590\3590X33

SEEDING	
END WIDTH	SQ. YDS.
156	
	889
164	
	939
174	
	1003
187	
	1067
197	
	1131
210	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 $\mathcal{L}_{CONST.}$ 20 40 60 80 100 120 140 160 180 200 220 240

1080 / 1060 / 1040

1.4%

MATCH LINE RAMP D

8:1

1059.2
80.80

1062.89

STATION 240+50.00

1066.40

1.6%

MATCH LINE RAMP C

1080 / 1060 / 1040

MATCH LINE RAMP D

1058.4
85.92

1062.73

STATION 240+00.00

1066.22

1.6%

MATCH LINE RAMP C

1080 / 1060 / 1040

MATCH LINE RAMP D

1057.6
91.12

1062.58

STATION 239+50.00

1066.71

1.6%

MATCH LINE RAMP C

1080 / 1060 / 1040

MATCH LINE RAMP D

1056.8
96.48

1062.45

STATION 239+00.00

1066.88

1.6%

MATCH LINE RAMP C

1080 / 1060 / 1040

MATCH LINE RAMP D

8:1

1056.0
102.00

1062.34

STATION 238+50.00

1064.02

0.2%

STA.238+00

STA.238+00

280 260 240 220 200 180 160 140 120 100 80 60 40 20 $\mathcal{L}_{CONST.}$ 20 40 60 80 100 120 140 160 180 200 220 240

END AREA		VOLUME	
CUT	FILL	CUT	FILL
1215	4		
		2457	4
1438	0		
		2630	0
1402	0		
		2263	0
1042	0		
		1509	0
587	0		
		731	8
202	8		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS

STA. 238+50.00 TO STA. 240+50.00

FAY-35-2.57

107
522

Q:\3590\3590X34

SEEDING END WIDTH	SEEDING SQ. YDS.
167	950
175	981
178	1023
190	1123
214 / 138	817
156	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 / 1060 / 1040

MATCH LINE RAMP D 0.40% 3.5% 48:1 6:1 1055.3 130.0'

1063.69
STATION 243+00.00
1056.00

8' 3' 12:1 4:1 0.2% PROP. LA R/W 6' 5' 4:1 1054.6 133.13'

1.4% MATCH LINE RAMP D 6:1 6:1 1056.0 111.6'

1063.53
STATION 242+50.00
1055.94

6' 5' 12:1 0.2% PROP. LA R/W 1054.7 132.98'

2.5% MATCH LINE RAMP D 1057.3 99.9'

1063.37
STATION 242+00.00
1057.51

6' 5' 12:1 0.2% PROP. LA R/W 1054.8 133.88'

2.5% MATCH LINE RAMP D 1058.6 88.2'

1063.21
STATION 241+50.00
1058.56

6' 5' 12:1 4:1 0.2% PROP. LA R/W 1054.9 135.49'

2.5% MATCH LINE RAMP D 6:1 8:1 1059.9 76.75'

BACK & AHEAD AHEAD

MATCH LINE RAMP C 12:1 4:1 4:1 0.2% PROP. LA R/W 1055.0 137.34'

1063.05
STATION 241+00.00
1063.83

1.4%

STA. 240+50 ℄ CONST. STA. 240+50

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
2	1466		
15	1190	16	2460
32	857	44	1896
125	456	146	1216
696	149	483	561
626	38	1705	39
1215	4		

AHEAD BACK

AHD BACK

CALCULATED J.L.G. CHECKED W.G.Y.

CROSS SECTIONS

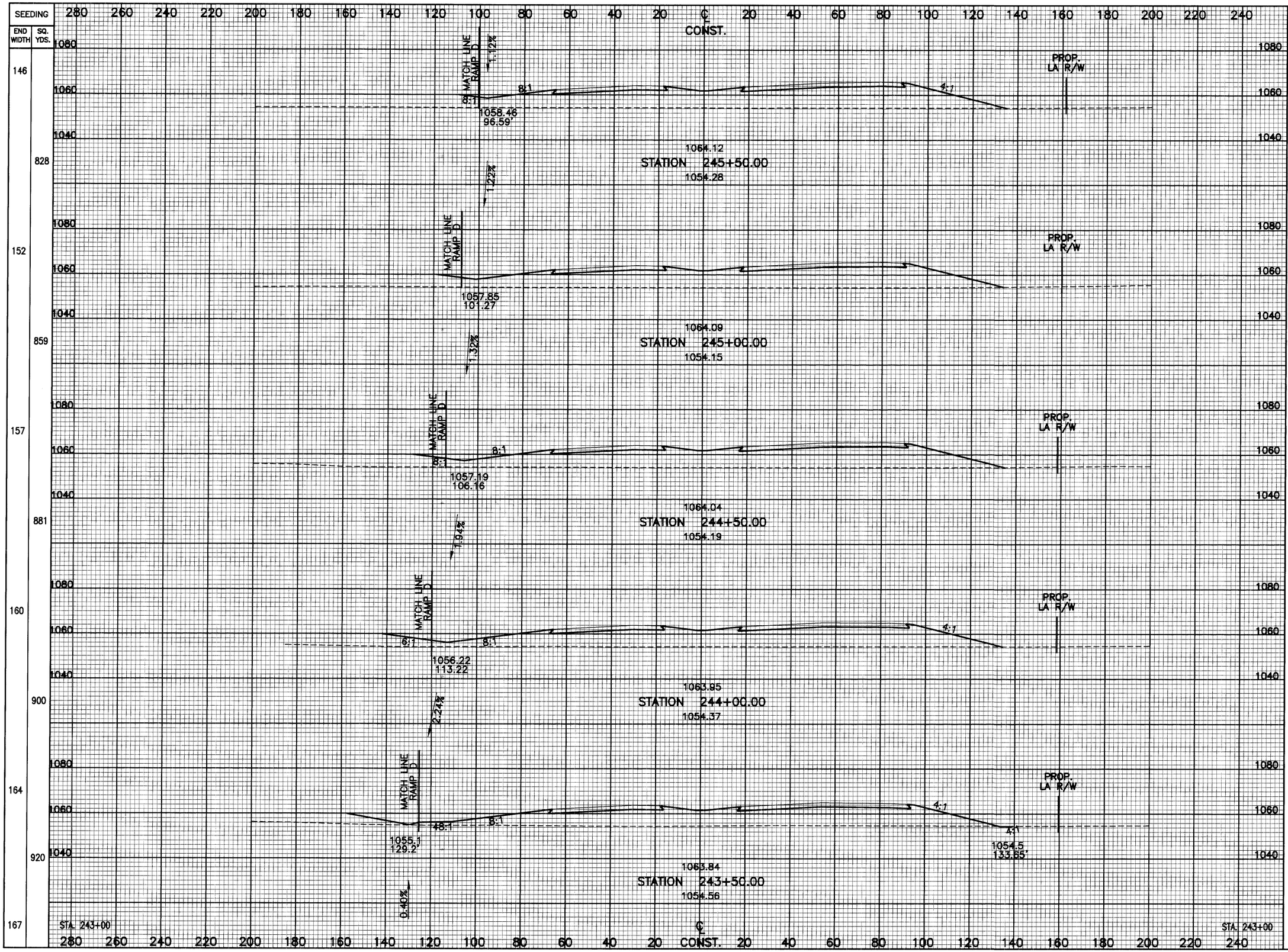
STA. 241+00.00 TO STA. 243+00.00

FAY-35-2.57

108
522

Q:\3590\3590X35

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
146	828	0	1744	0	3258	
152	859	0	1774	0	3288	
157	881	0	1776	0	3234	
160	900	0	1716	1	3087	
164	920	1	1617	3	2855	
167		2	1466			

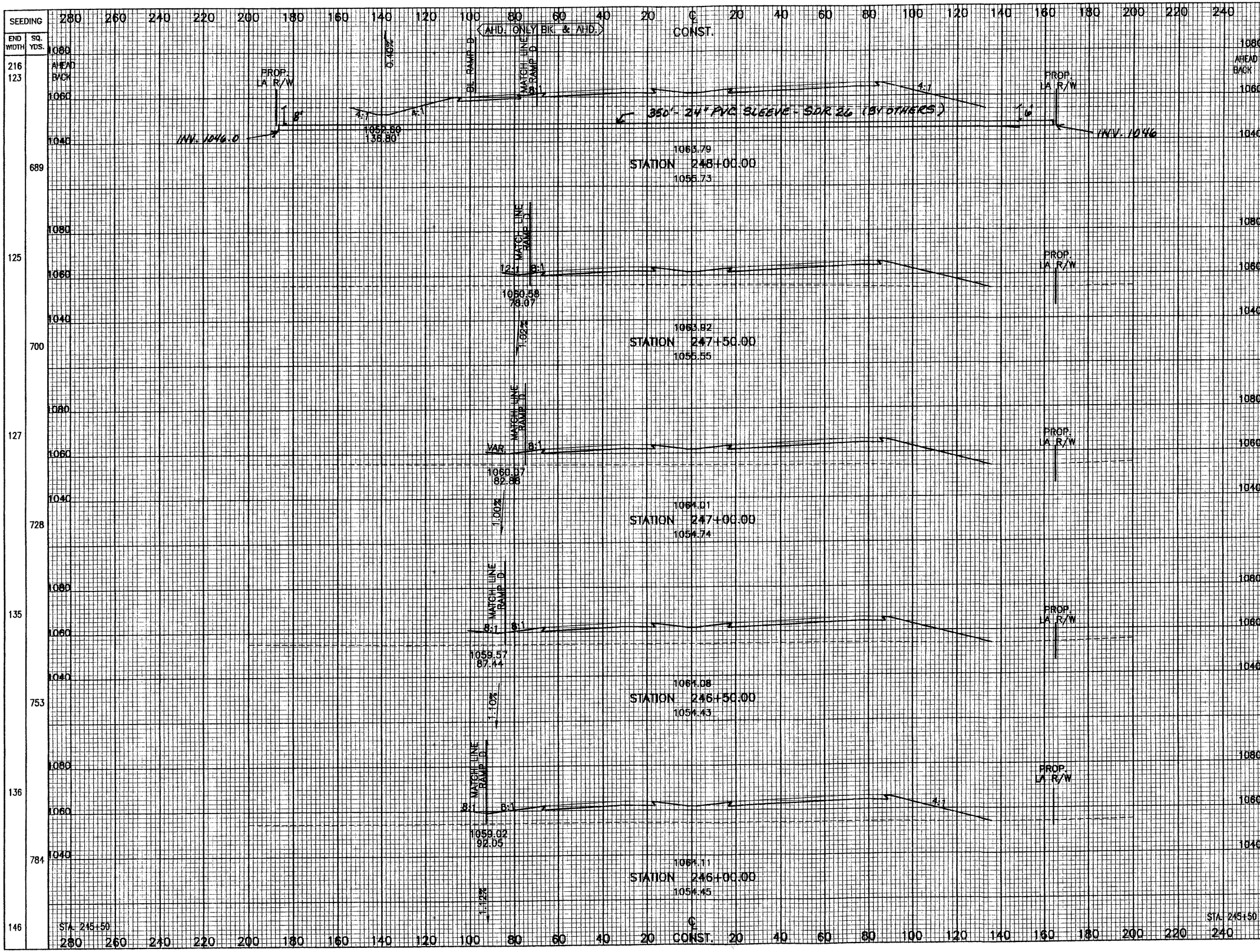


CROSS SECTIONS
STA. 243+50.00 TO STA. 245+50.00

FAY-35-2.57

109
522

O:\3590\3590X36



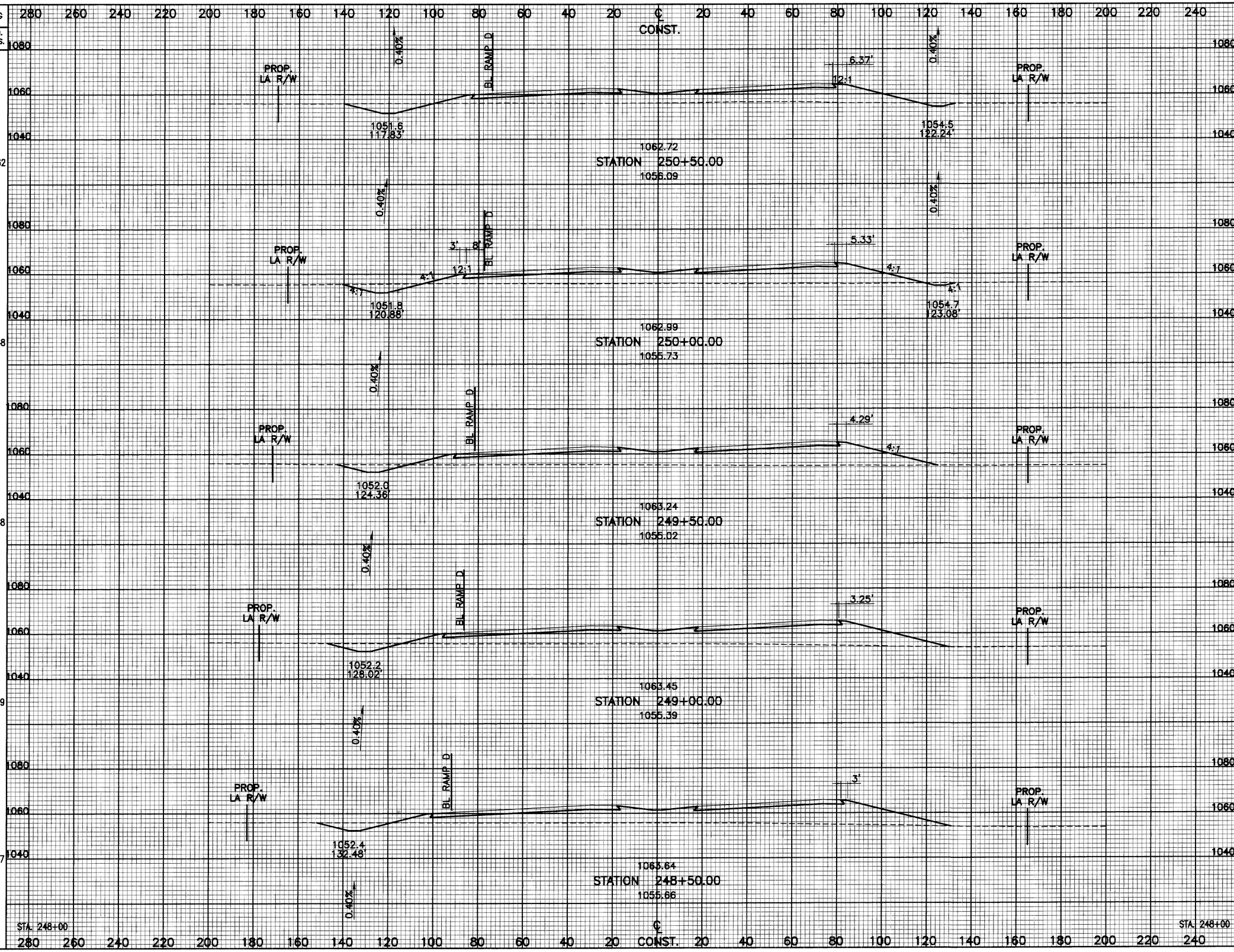
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
57	1469		
0	1304		
		0	2495
0	1390		
		0	2713
0	1540		
		0	2947
0	1642		
		0	3079
0	1683		
		0	3174
0	1744		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 246+00.00 TO STA. 248+00.00

FAY-35-2.57

Q:\3590\3590X37



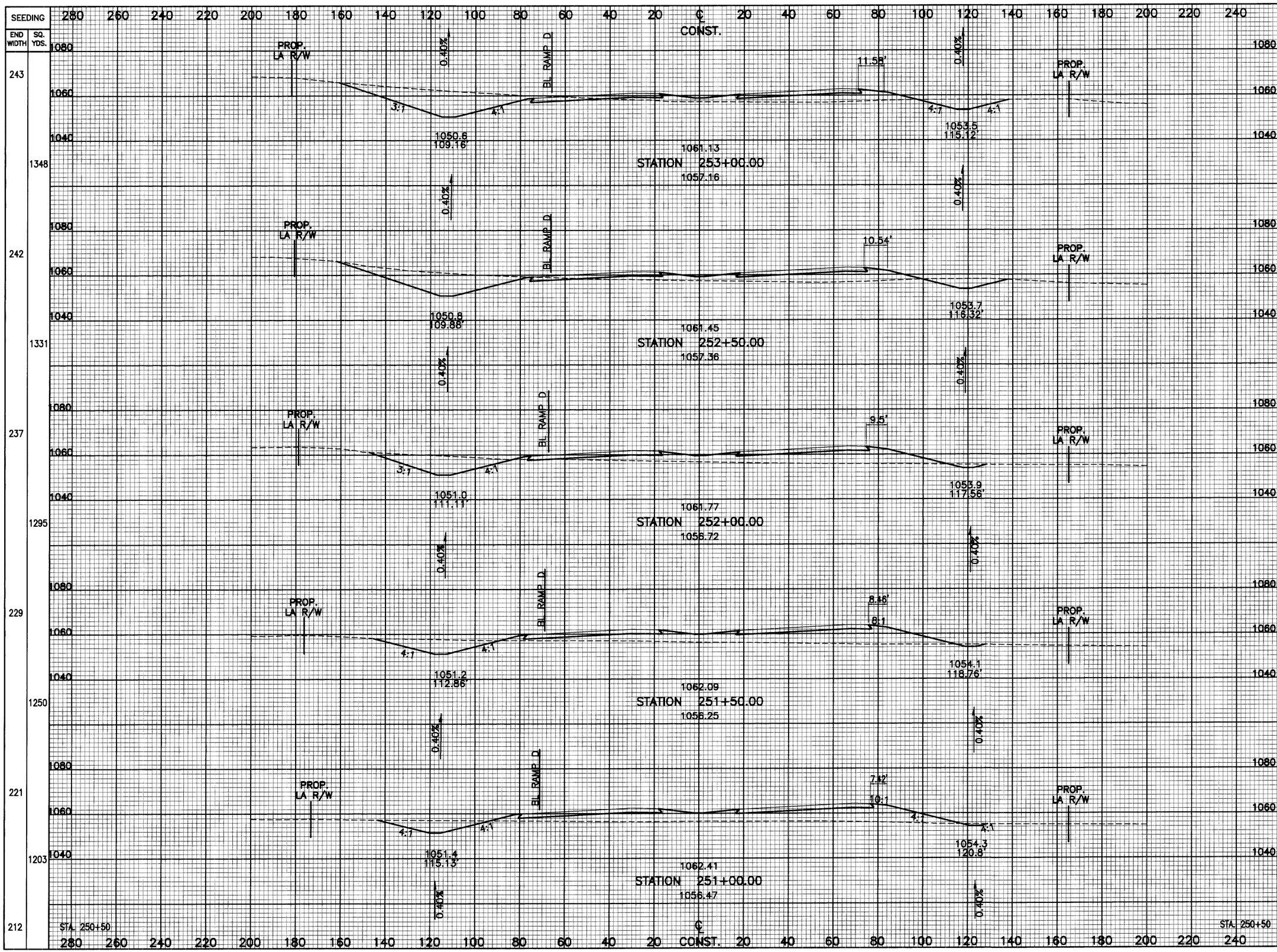
SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
212		114	957		
	1162			187	1936
206		87	1133		
	1148			138	2318
207		61	1370		
	1148			117	2570
206		65	1405		
	1139			123	2596
204		67	1398		
	1167			115	2655
216		57	1469		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 248+50.00 TO STA. 250+50.00

FAY-35-2.57

Q:\3590\3590X38



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
693	353		
		1153	720
552	424		
		800	977
312	631		
		479	1328
205	803		
		338	1534
160	852		
		254	1675
114	957		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 251+00.00 TO STA. 253+00.00

FAY-35-2.57

Q:\3590\3590X39

SEEDING	
END WIDTH	SQ. YDS.
244	1353
243	1356
245	1353
242	1348
243	1494
243	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 / 1060 / 1040

PROP. LA R/W

0.80% 4.79' 8.48' 12:1 4:1 4:1 1049.4 108.09' 1059.53 STATION 255+50.00 1051.94 1.80% 16.79' 4:1 4:1 1049.7 120.28' PROP. LA R/W

0.80% 3' 8' BL RAMP D 12:1 4:1 4:1 1049.8 107.27' 1059.85 STATION 255+00.00 1053.19 1.80% 15.75' 4:1 4:1 1050.6 116.68' PROP. LA R/W

0.40% BL RAMP D CATCH BASIN NO 4 STA 254+50, ℄ GRATE EL=1067.69 IL 15" P-26=1053.25 C-26 HEADWALL HW-2.1M OR 2.2M STA 254+50, 115' RT IL 15" P-26=1051.50 H-26 1.80% 14.71' 4:1 4:1 1050.0 107.72' 1060.17 STATION 254+50.00 1054.47 1051.5 117.08' RCP, TYPE C PROP. LA R/W

0.40% BL RAMP D 3.80% 13.67' 3:1 4:1 1050.2 108.20' 1060.49 STATION 254+00.00 1055.93 1052.4 115.48' PROP. LA R/W

0.40% BL RAMP D 1.80% 12.62' 3:1 4:1 4:1 4:1 1050.4 108.68' 1060.81 STATION 253+50.00 1056.47 1053.3 113.92' PROP. LA R/W

0.40% 0.40% STA.253+00 STA.253+00

℄ CONST.

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
192	958		
		453	1650
297	824		
		675	1356
431	640		
		948	1050
592	494		
		1170	834
671	406		
		1263	703
693	353		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS

STA. 253+50.00 TO STA. 255+50.00

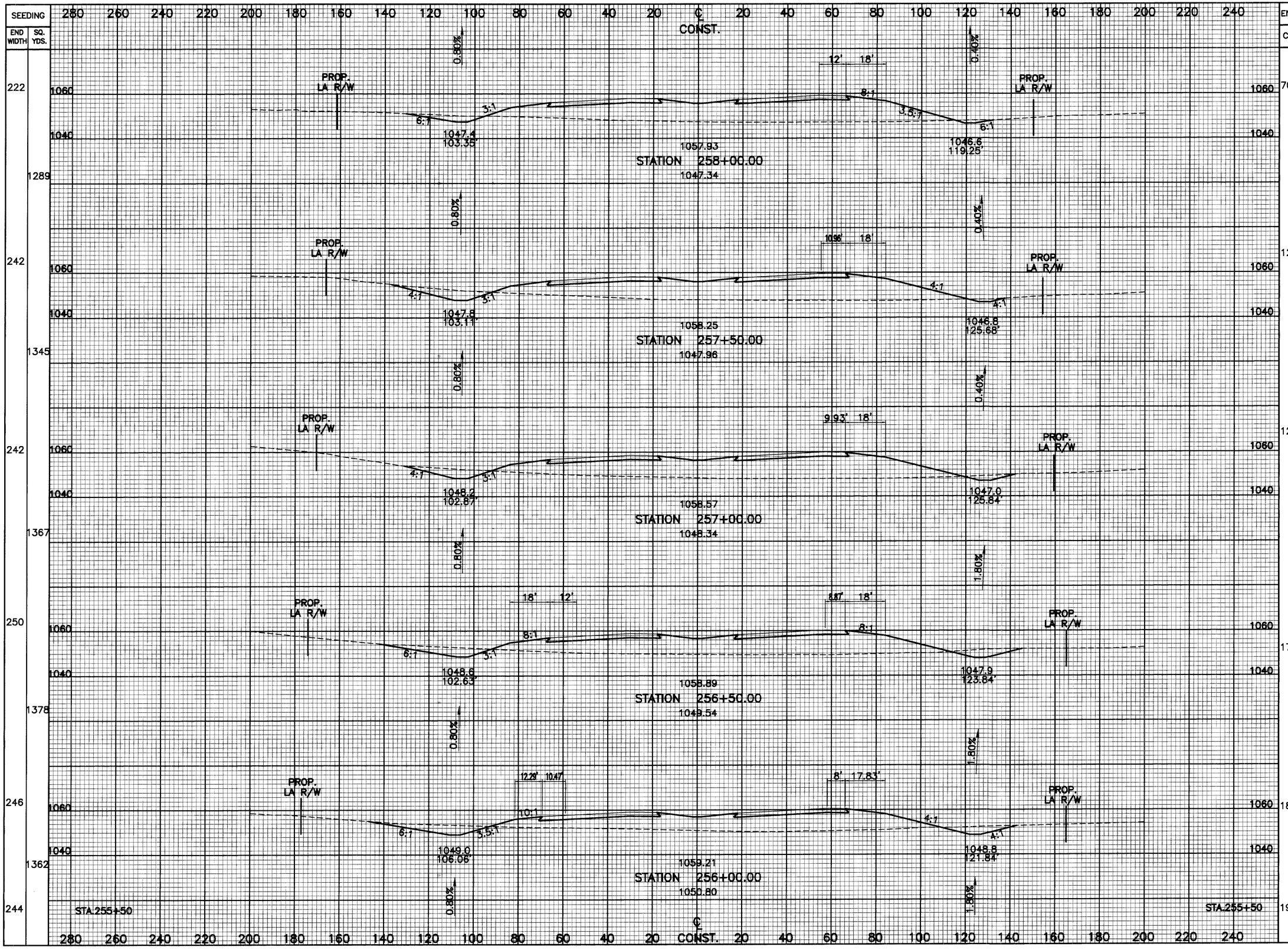
FAY-35-2.57

113
522

Q:\3590\3590X40

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
222	1289	70	1576	177	2896
242	1345	121	1551	225	2860
242	1367	122	1537	277	2688
250	1378	177	1365	337	2346
246	1362	186	1168	350	1969
244		192	958		

CALCULATED J.L.G.
CHECKED W.G.Y.



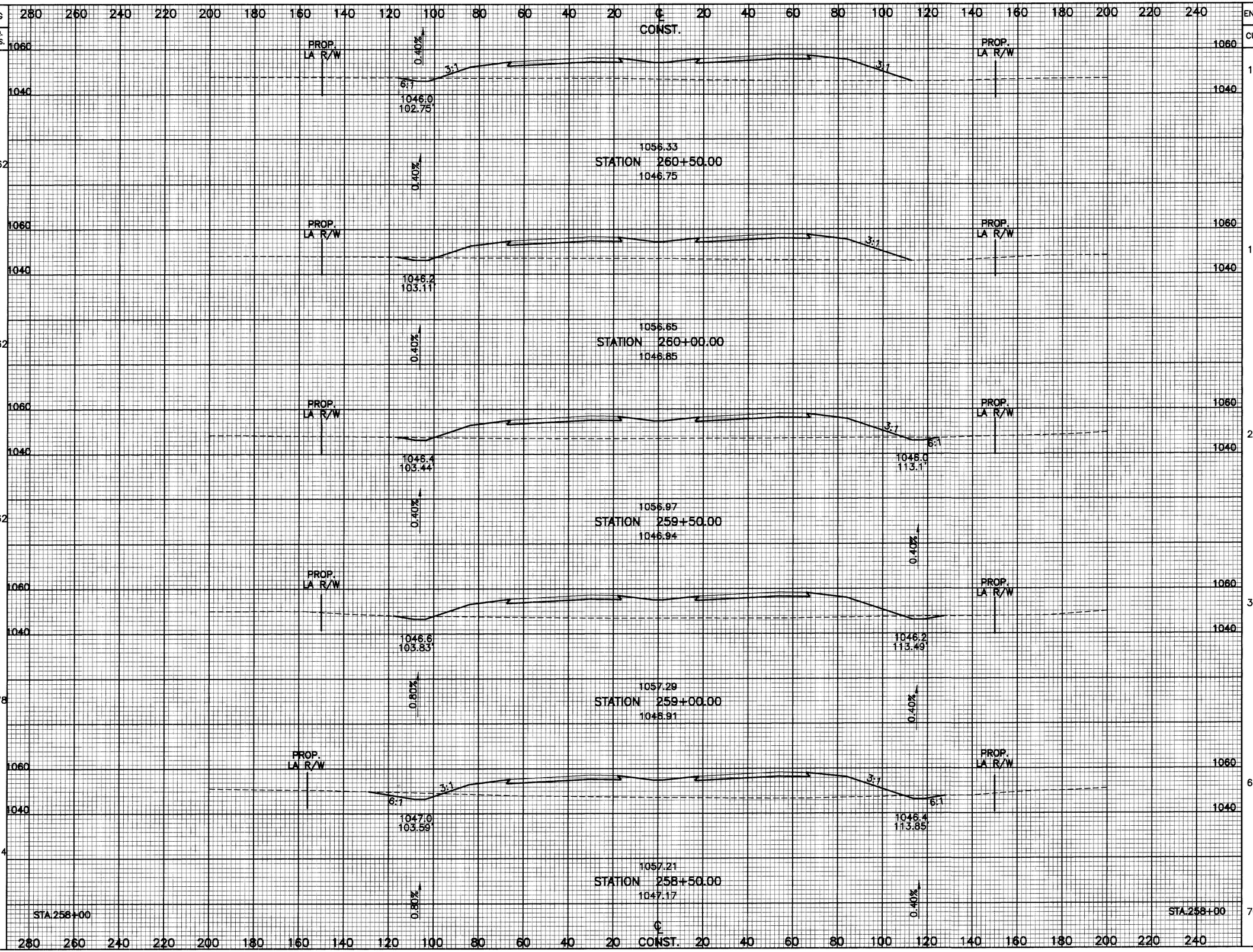
CROSS SECTIONS
STA. 256+00.00 TO STA. 258+00.00

FAY-35-2.57

114
522

Q:\3590\3590X41

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
209		16	1498			
	1162			28	2821	
209		14	1548			
	1162			36	2847	
209		24	1526			
	1162			51	2875	
209		31	1578			
	1178			93	2851	
215		69	1501			
	1214			129	2850	
222		70	1576			



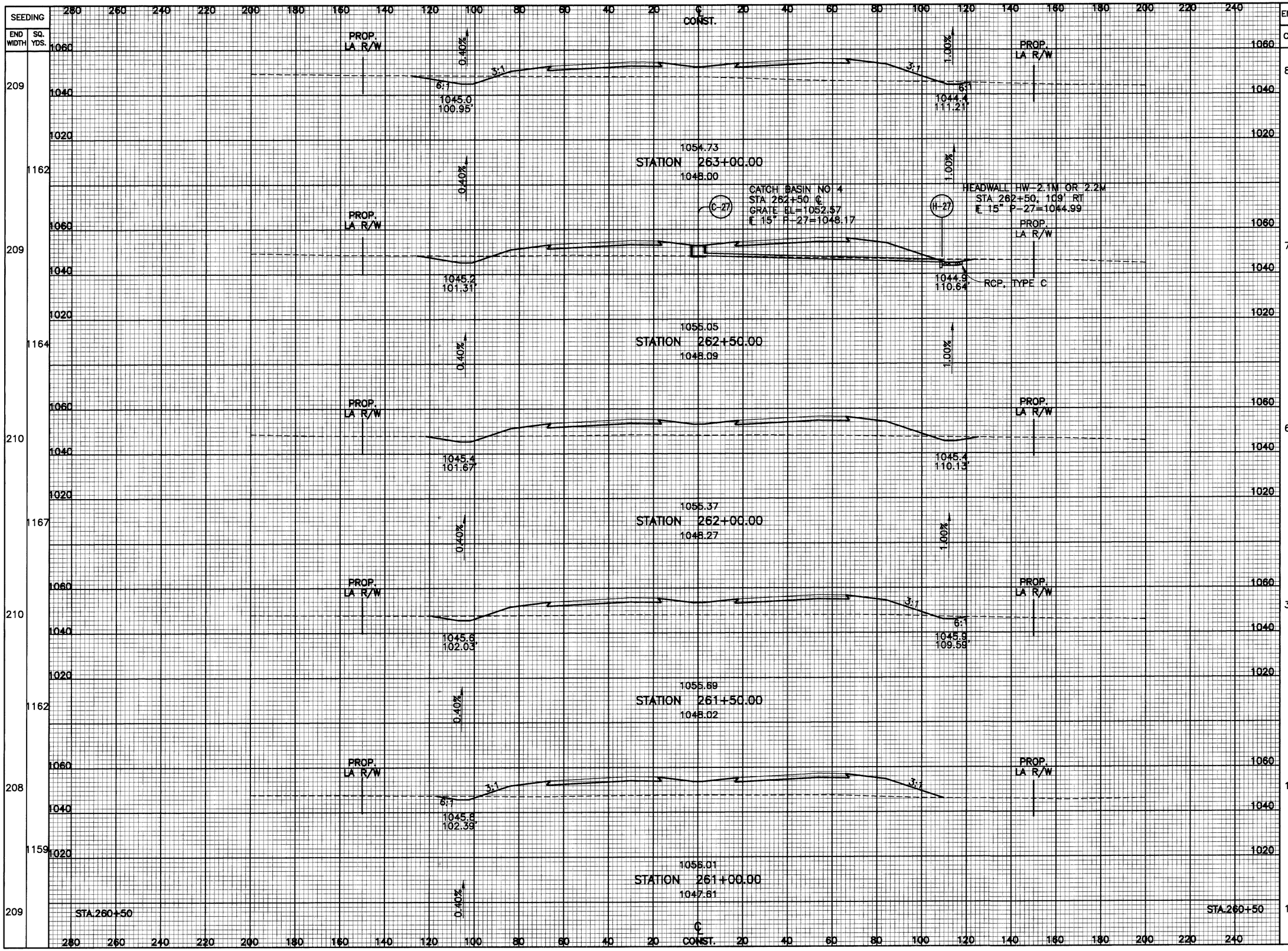
CROSS SECTIONS
STA. 258+50.00 TO STA. 260+50.00

FAY-35-2.57

115
522

Q:\3590\3590X42

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.	CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL		
209	1162	87	1013	150	1866		
209	1164	75	1002	130	1850		
210	1167	65	996	96	1936		
210	1162	38	1094	52	2200		
208	1159	18	1281	32	2574		
209		16	1498				



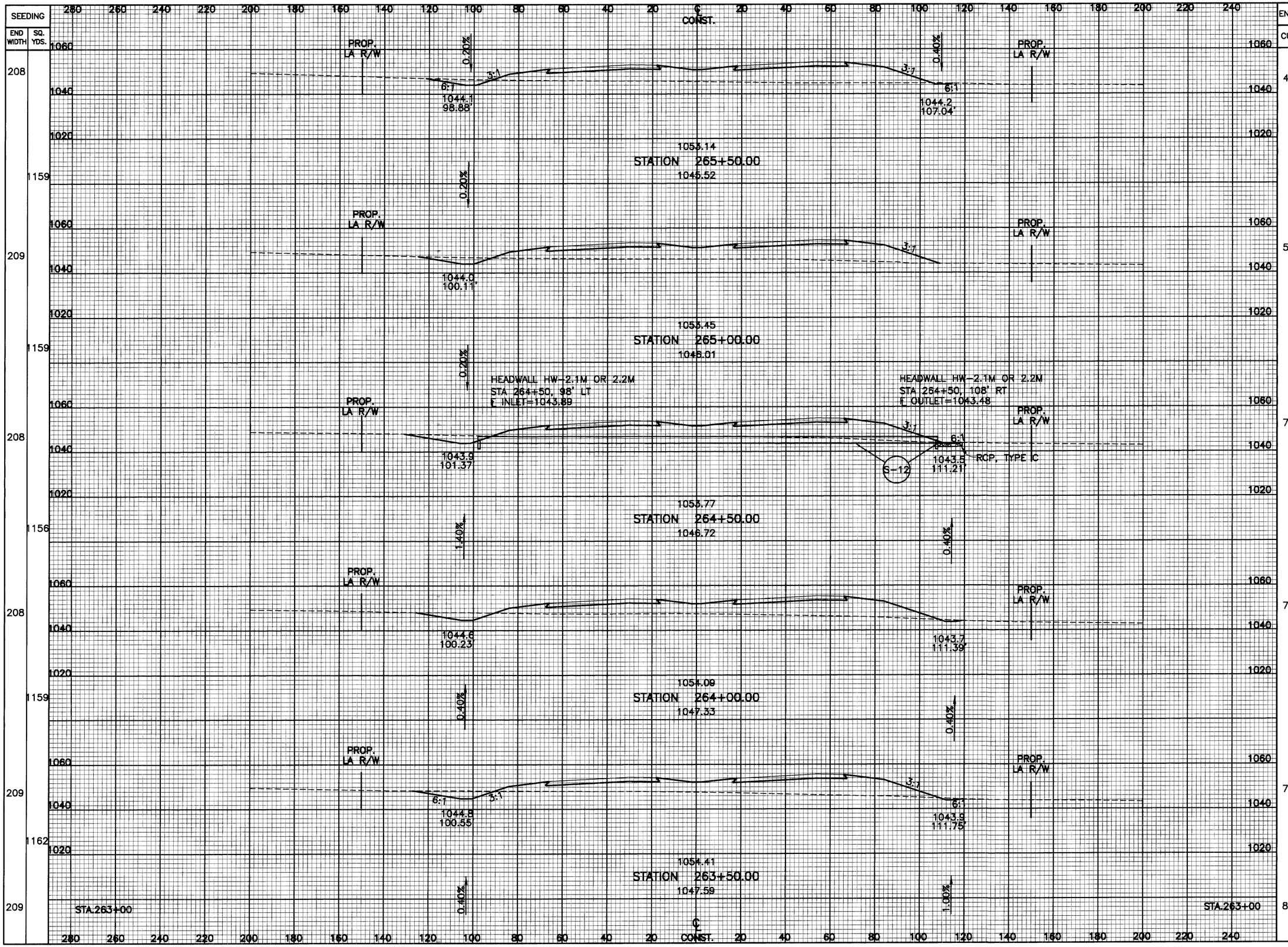
CROSS SECTIONS
STA. 261+00.00 TO STA. 263+00.00

FAY-35-2.57

116
522

O:\3590\3590X43

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
208	1159	46	1079	92	1988	
209	1159	53	1068	123	1953	
208	1156	79	1041	141	1880	
208	1159	73	989	139	1834	
209	1162	77	991	152	1856	
209		87	1013			



CROSS SECTIONS
STA. 263+50.00 TO STA. 265+50.00

FAY-35-2.57

Q:\3590\3590X44

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
207	1145	68	562	116	1096	
205	1142	57	621	109	1254	
206	1145	60	733	121	1444	
206	1148	70	826	118	1654	
207	1153	57	960	96	1888	
208		46	1079			

℄ CONST.

PROP. LA R/W

0.20%

6:1 3:1

1044.6 93.09'

1046.0 97.35'

1.12%

1051.71
STATION 268+00.00
1046.26

BEGIN DITCH +55

1044.5 94.2'

1046.5 96.66'

END DITCH +45

1051.98
STATION 267+50.00
1046.43

1.56%

1044.4 95.31'

1045.8 99.57'

1052.25
STATION 267+00.00
1045.88

1.40%

1044.3 96.48'

1045.1 102.54'

1052.54
STATION 266+50.00
1045.91

1044.2 97.65'

1044.4 105.51'

1052.83
STATION 266+00.00
1045.60

0.40%

STA.265+50

1060 1040 1020

CROSS SECTIONS
STA. 266+00.00 TO STA. 268+00.00

FAY-35-2.57

118
522

O:\3590\3590X45

SEEDING	
END WIDTH	SQ. YDS.
208	1156
208	1156
208	1156
208	1153
207	1150
207	

END AREA		VOLUME		CALCULATED J.L.G.	CHECKED W.G.Y.
CUT	FILL	CUT	FILL		
73	523	127	964		
64	518	107	972		
51	531	98	1018		
54	568	110	1020		
64	533	123	1014		
68	562				

HEADWALL HW-2.1M OR 2.2M CONST.
STA 270+65, 90' LT
℄ 24" P-41=1044.34

CATCH BASIN NO 4
STA 270+65, ℄
GRATE EL=1047.72
℄ 24" P-41,P-42=1044.18

HEADWALL HW-2.1M OR 2.2M
STA 270+65, 96' RT
℄ 24" P-42=1044.01

RCP TYPE C

1057.93
STATION 270+50.00
1045.73

1050.75
STATION 270+00.00
1046.00

1050.98
STATION 269+50.00
1045.99

1051.21
STATION 269+00.00
1045.85

1051.46
STATION 268+50.00
1046.20

BEGIN DITCH +85

END DITCH +65

PROP. LA R/W

1044.4 92.01'
1044.8 90.84'
1044.9 90.45'
1044.8 90.99'
1044.7 92.04'
1044.1 98.82'
1044.2 100.11'
1045.2 98.25'
1045.6 97.8

0.80%
0.20%
2.00%

STA.268+00

℄ CONST.

CROSS SECTIONS
STA. 268+50.00 TO STA. 270+50.00

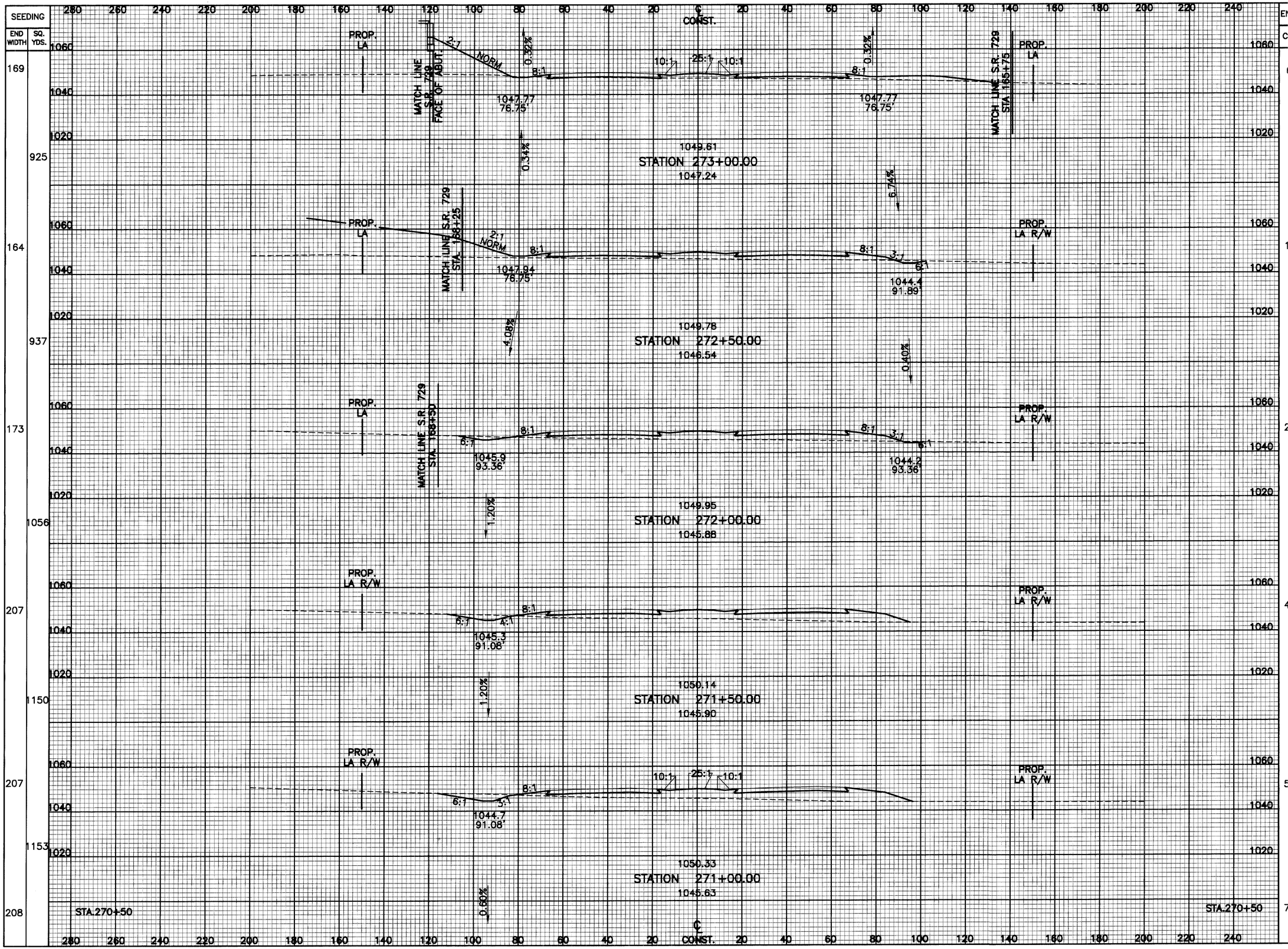
FAY-35-2.57

119
522

O:\3590\3590X46

SEEDING	
END WIDTH	SQ. YDS.
169	925
164	937
173	1056
207	1150
207	1153
208	

END AREA		VOLUME		CALCULATED J.L.G.
CUT	FILL	CUT	FILL	CHECKED W.G.Y.
6	471			
		15	786	
10	377			
		31	725	
23	405			
		59	841	
40	503			
		91	980	
58	555			
		122	999	
73	523			



CROSS SECTIONS STA. 271+00.00 TO STA. 273+00.00

FAY-35-2.57

120
522

Q:\3590\3590X47

SEEDING	
END WIDTH	SQ. YDS.
206	2289
206	2050
163	1856
171	
146	812
146	875
169	

END AREA		VOLUME		CALCULATED	J.L.G.
CUT	FILL	CUT	FILL	CHECKED	W.G.Y.
34	509				
		125	1493		
33	297				
		136	999		
40	242				
		102	1397		
15	512				
		61	1205		
50	789				
		52	1167		
6	471				

CROSS SECTIONS

STA. 273+50.00 TO STA. 277+00.00

FAY-35-2.57

121
522

Q:\3590\3590X48

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
206					
	2295	13	756	73	3210
207					
	2306	26	977	63	3717
208					
	2306	8	1030	99	3421
207					
	2306	45	817	152	2725
208					
	2300	37	654	132	2154
206		34	509		

CALCULATED J.L.G.
CHECKED W.G.Y.

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1060 / 1040 / 1020

PROP. LA R/W

ELEV. 1042 — 5' — 4:1 — 4:1 — 6:1

0.20%

1038.8
104.72'

1046.73
STATION 282+00.00
1040.84

0.20%

2.00%

8:1 — 3:1 — 6:1

1041.7
90.84'

1.92%

3-D
STA 281+14, 145' LT
18" CONDUIT, TYPE B
℄ OUTLET=1036.70

PROP. LA R/W

ELEV. 1042 — 5' — 4:1 — 4:1 — 6:1

3-D

1039.0
105.20'

RCP, TYPE B

1039.78
97.56'

S-14
HEADWALL HW-2.2M
STA 280+94.75, 94.83' LT
℄ OUTLET=1039.07

1047.05
STATION 281+00.00
1039.91

0.40%

H-28
HEADWALL HW-2.1M OR 2.2M
STA 280+00.98' LT
℄ 15" P-28=1039.44

C-28
CATCH BASIN NO 4
STA 280+00, ℄
GRATE EL=1044.62
℄ 15" P-28=1040.11

0.20%

3-D

PROP. LA R/W

ELEV. 1042 — 5' — 4:1 — 6:1 — 3:1

RCP, TYPE C

1039.4
99.66'

S-14 — 3-D

1039.56
99.12'

S-14
HEADWALL HW-2.2M
STA 279+85.25, 94.83' RT
℄ INLET=1039.50

1047.37
STATION 280+00.00
1039.96

2.00%

3-D
STA 279+45, 145' RT
18" CONDUIT, TYPE B
℄ INLET=1037.12

PROP. LA R/W

1041.4
94.62'

1039.68
99.78'

1047.69
STATION 279+00.00
1040.97

0.20%

0.24%
2.0%
DITCH BREAK -50

PROP. LA R/W

6:1 — 3:1 — 8:1 — 3:1 — 6:1

1042.6
91.98'

1040.6
97.38'

1048.01
STATION 278+00.00
1042.15

0.40%

2.00%

STA.277+00

STA.277+00

℄ CONST.

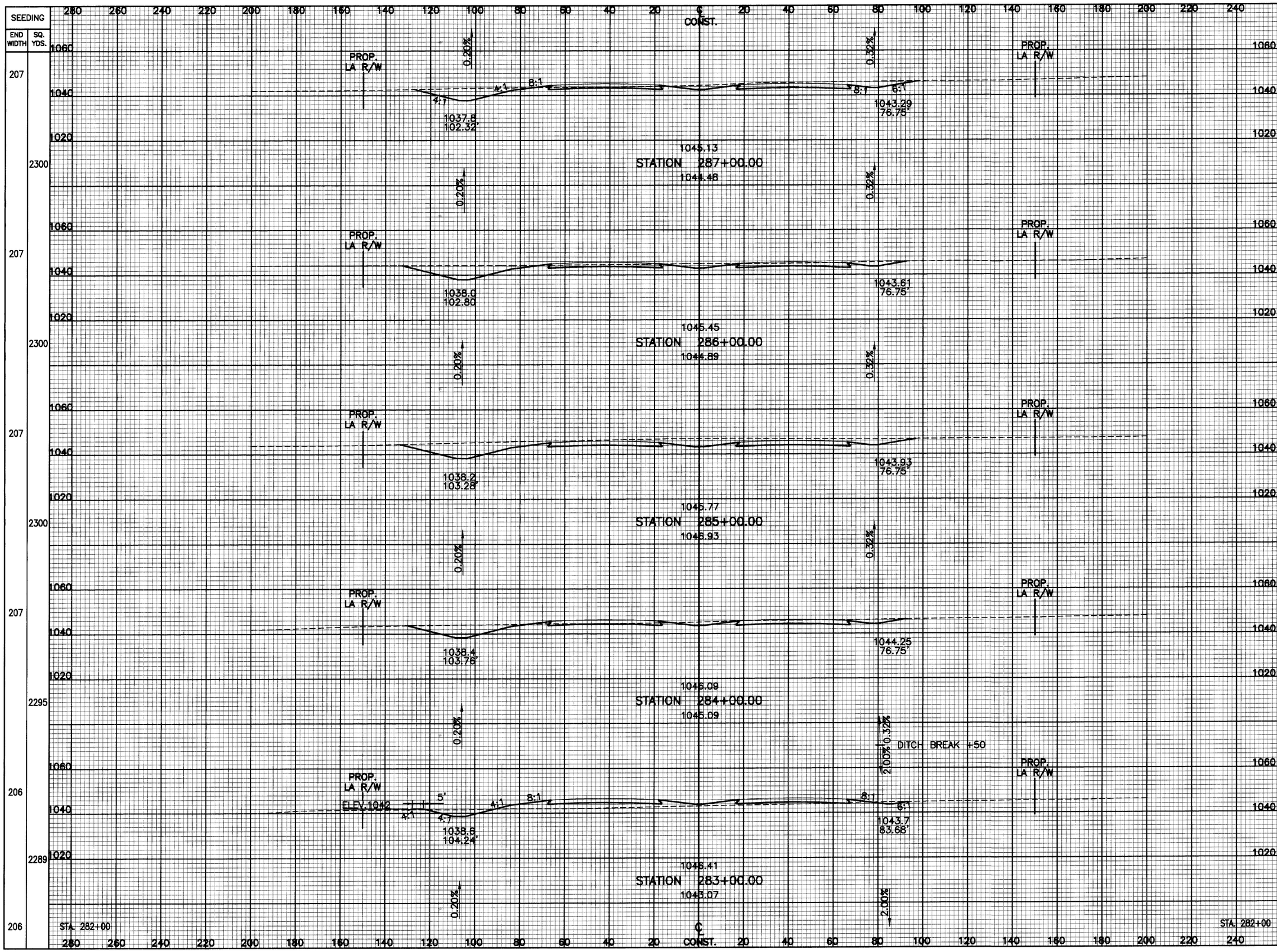
280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

CROSS SECTIONS
STA. 278+00.00 TO STA. 282+00.00

FAY-35-2.57

122
522

Q:\3590\3590X49



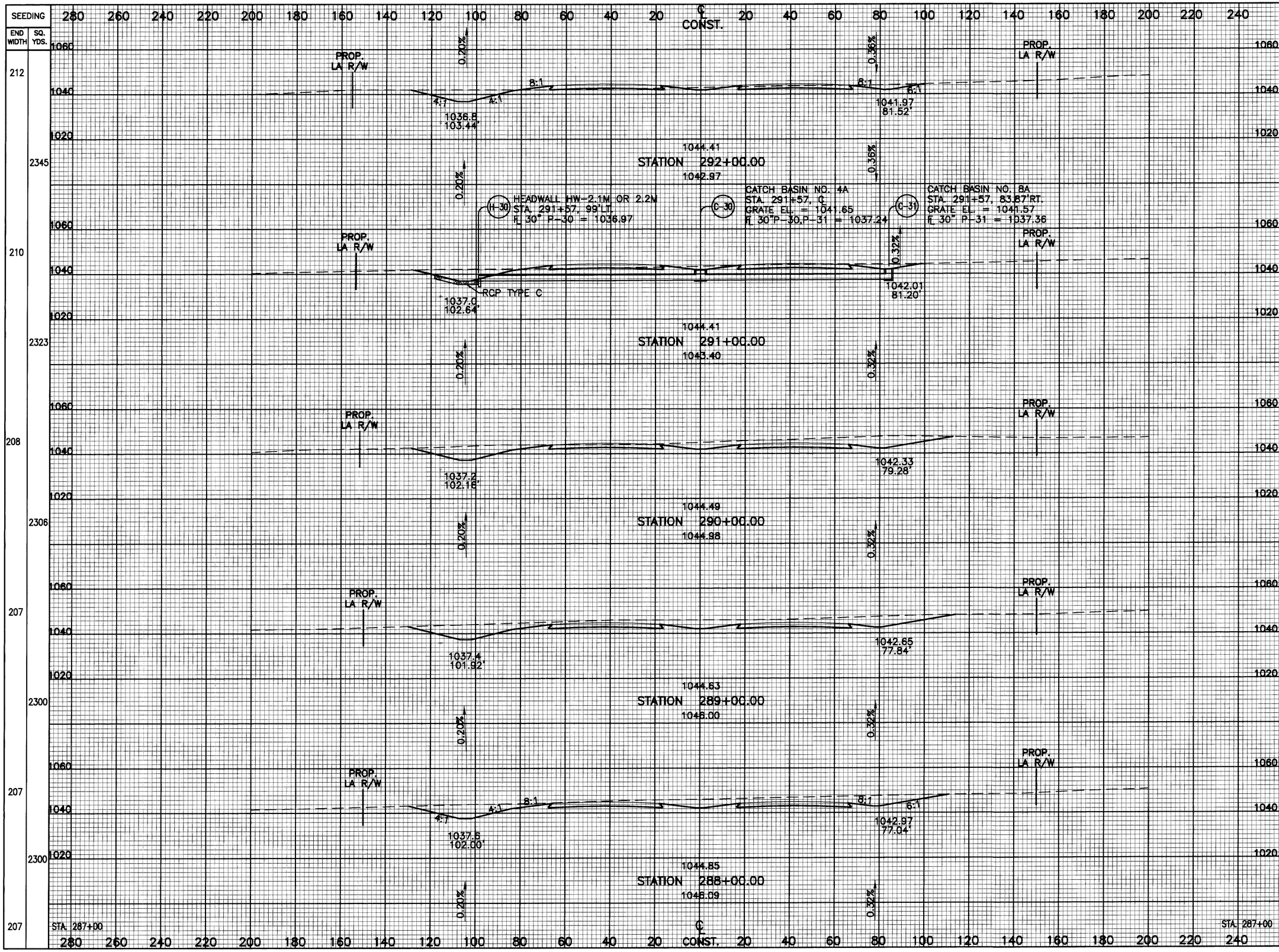
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
364	5		
		1388	13
385	2		
		1943	4
664	0		
		1802	15
309	8		
		686	512
61	268		
		138	1897
13	756		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 283+00.00 TO STA. 287+00.00

FAY-35-2.57

Q:\3590\3590X50



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
249	30		
		1019	76
301	11		
		1830	21
687	0		
		2745	0
795	0		
		2852	0
745	0		
		2054	10
364	5		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 288+00.00 TO STA. 292+00.00

FAY-35-2.57

Q:\3590\3590X51

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
217	1209	180	145	342	186	
218	1209	189	55	425	59	
217	1203	270	8	570	12	
216	2384	345	4	1145	30	
213	2417	273	12	967	78	
212		249	30			

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1060 / 1040 / 1020

PROP. LA R/W

0.10%

4:1 4:1

1036.1
109.92'

8:1 8:1 6:1

0.80%

1041.2
88.64'

1044.93
STATION 295+50.00
1042.16

0.20%

1.00%

PROP. LA R/W

1036.2
108.24'

1042.0
82.88'

1044.81
STATION 295+00.00
1042.82

0.20%

1.60%

PROP. LA R/W

1036.3
106.6'

1042.86
76.75'

1044.70
STATION 294+50.00
1043.66

0.20%

0.34%

PROP. LA R/W

1036.4
105.84'

1042.69
77.36'

1044.61
STATION 294+00.00
1043.84

0.20%

0.36%

PROP. LA R/W

4:1 4:1

1036.6
104.48'

8:1 8:1 6:1

1042.33
79.12'

1044.47
STATION 293+00.00
1043.29

0.20%

0.36%

STA. 292+00

STA. 292+00

℄ CONST.

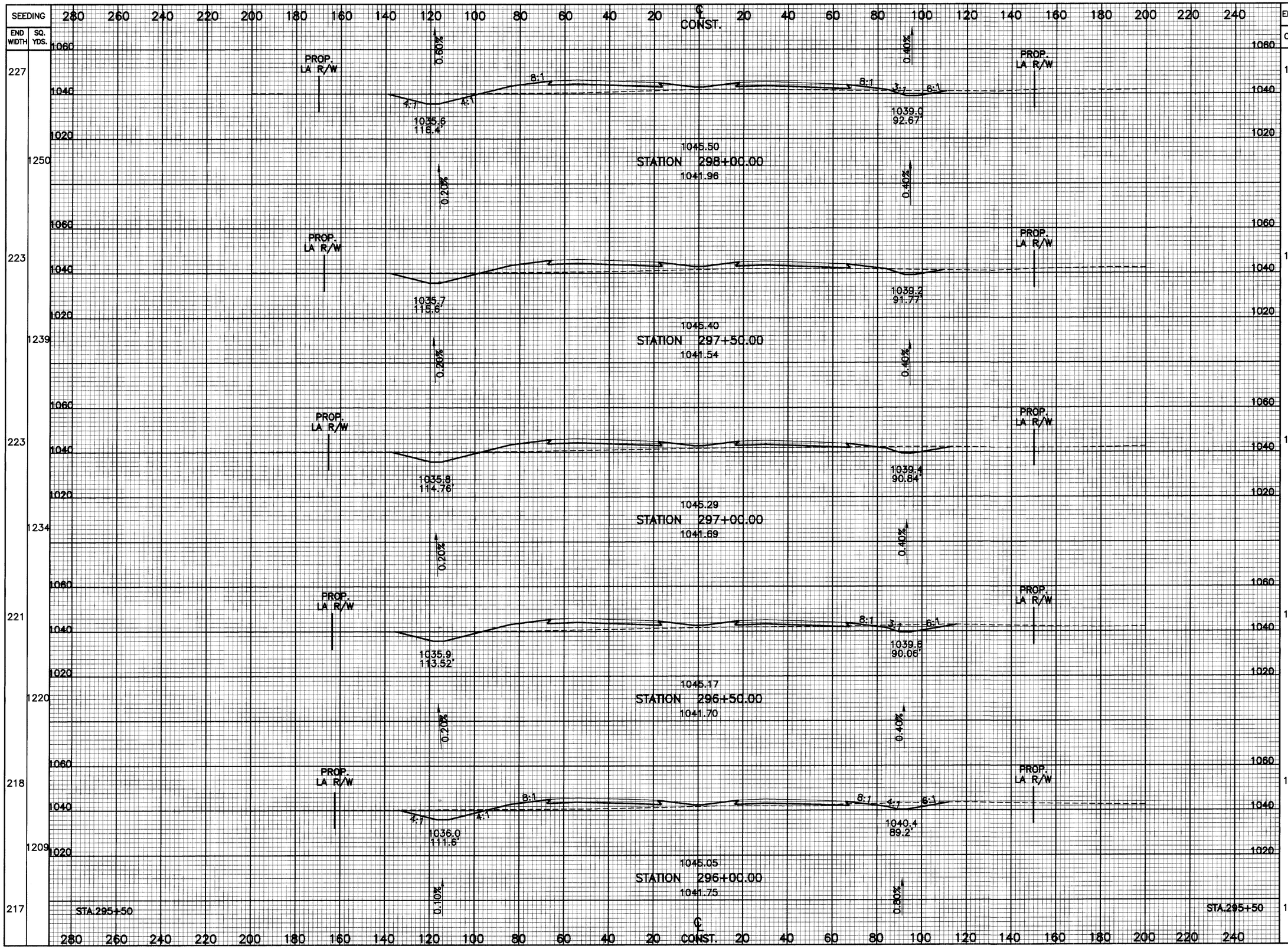
CROSS SECTIONS

STA. 293+00.00 TO STA. 295+50.00

FAY-35-2.57

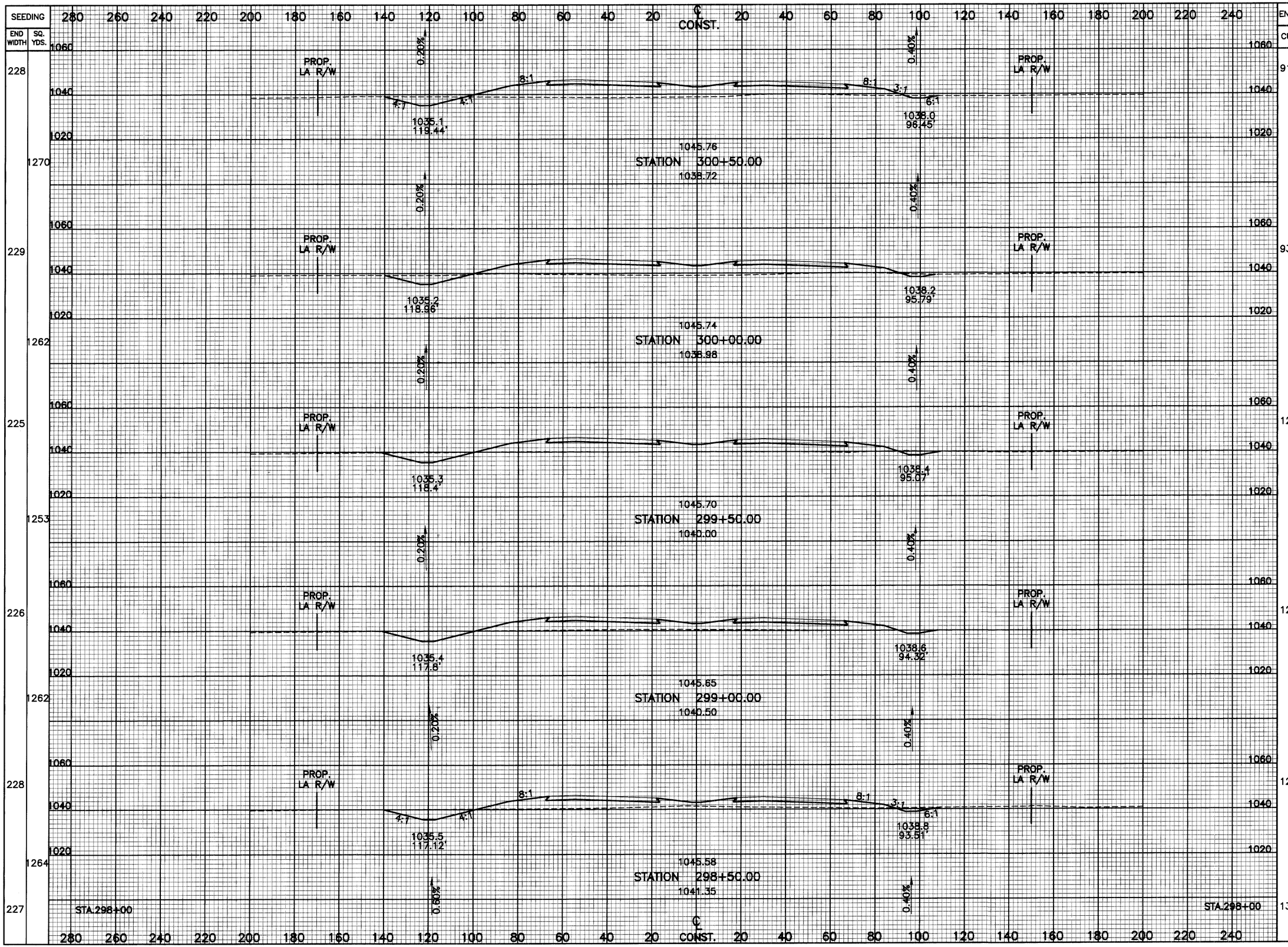
125
522

Q:\3590\3590X52


SEEDING
END WIDTH
SQ. YDS.
227
1250
223
1239
223
1234
221
1220
218
1209
217
STA.295+50
℄ CONST.
PROP. LA R/W
0.60%
0.40%
0.20%
0.40%
6:1
8:1
4:1
1035.6
116.4
1039.0
92.67
1045.50
STATION 298+00.00
1041.96
1035.7
115.6
1039.2
91.77
1045.40
STATION 297+50.00
1041.54
1035.8
114.76
1039.4
90.84
1045.29
STATION 297+00.00
1041.69
1035.9
113.52
1039.8
90.06
1045.17
STATION 296+50.00
1041.70
1036.0
111.6
1040.4
89.2
1045.05
STATION 296+00.00
1041.75
0.10%
0.80%
END AREA
CUT
FILL
139 412
137 397
158 347
160 328
158 292
180 145
VOLUME
CUT
FILL
256 750
274 689
295 625
295 575
313 405
CALCULATED
J.L.G.
CHECKED
W.G.Y.
CROSS SECTIONS
STA. 296+00.00 TO STA. 298+00.00
FAY-35-2.57
126
522
O:\3590\3590X53

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
228	1270	91	888	171	1563
229	1262	93	799	202	1381
225	1253	125	692	231	1220
226	1262	124	625	235	1075
228	1264	129	535	249	877
227		139	412		

CALCULATED J.L.G.
CHECKED W.G.Y.



CROSS SECTIONS

STA. 298+50.00 TO STA. 300+50.00

FAY-35-2.57

127
522

Q:\3590\3590X54

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
254	1414	303+00.00	122	977	217	1800
255	1412	302+50.00	112	967	209	1750
253	1406	302+00.00	113	923	201	1702
253	1403	301+50.00	104	915	181	1679
252	1334	301+00.00	91	898	169	1654
228		STA.300+50	91	888		

CALCULATED J.L.G.
CHECKED W.G.Y.

PROP. LA R/W
EX. 6" TILE
0.20%
0.40%
8:1
4:1
3:1
6:1
1034.8 120.5'
1036.0 102.12'
1045.65
STATION 303+00.00
1038.50
1034.7 120.8'
1036.2 101.67'
1045.70
STATION 302+50.00
1038.64
1034.8 120.56'
1036.2 101.67'
1045.70
STATION 302+00.00
1038.91
1034.9 120.24'
1037.1 99.15'
1045.76
STATION 301+50.00
1038.64
1035.0 119.58'
1037.8 97.08'
1045.77
STATION 301+00.00
1039.60

(18-D) 8" COND. TYPE F
STA.301+75, 105.5'RT.
℄ OUTLET = 1036.8

(18-D) 8" COND. TYPE F
STA.301+75, 144.5'RT.
℄ OUTLET = 1037.2

STA.300+50

℄ CONST.

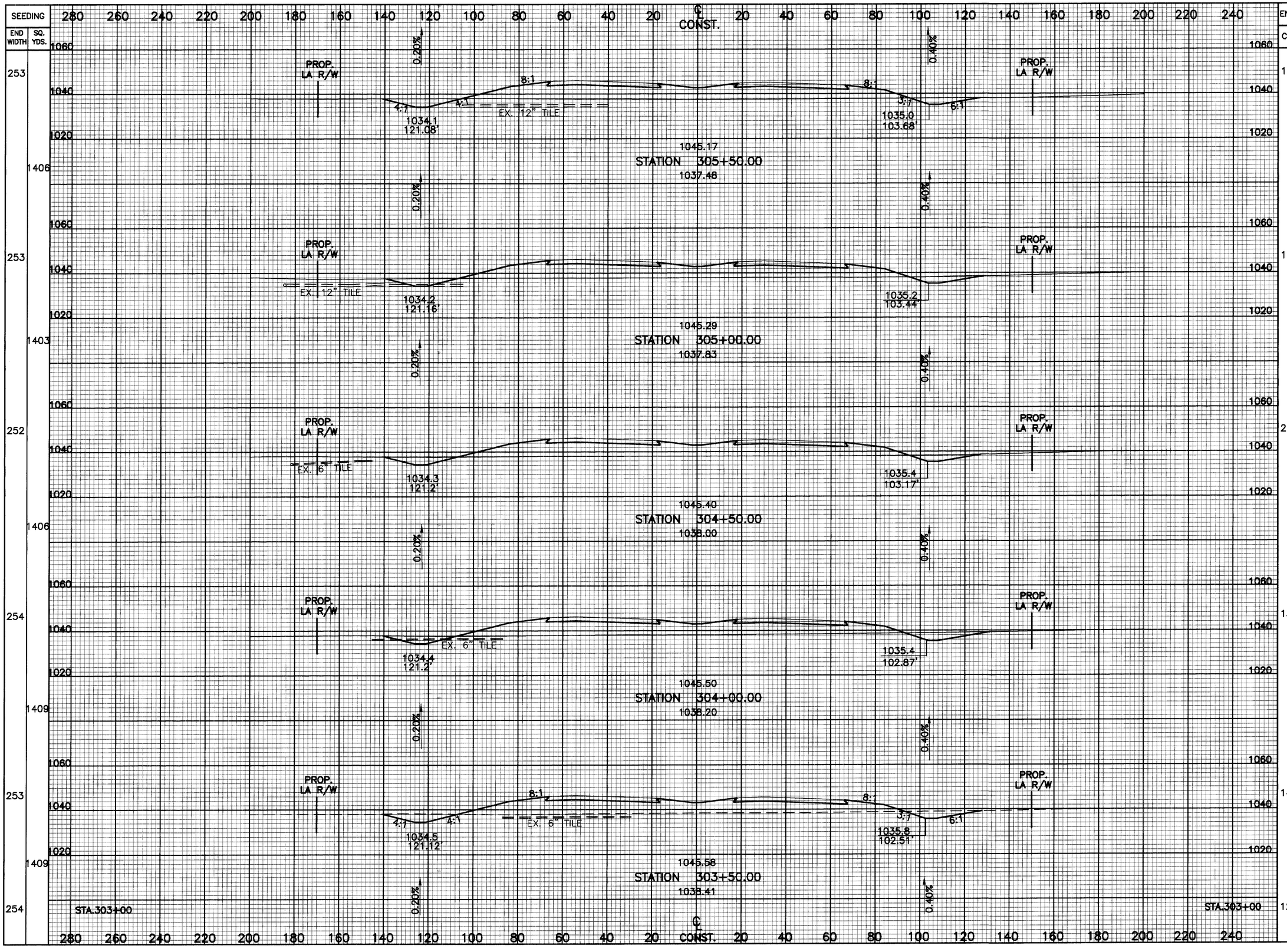
CROSS SECTIONS
STA. 301+00 TO STA. 303+00.00

FAY-35-2.57

128
522

O:\3590\3590X55

SEEDING END WIDTH	SEEDING SQ. YDS.
253	1406
253	1403
252	1406
254	1409
253	1409
254	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
130	1046		
		229	1921
117	1028		
		212	1903
212	1027		
		224	1872
130	994		
		255	1840
145	993		
		248	1825
122	977		

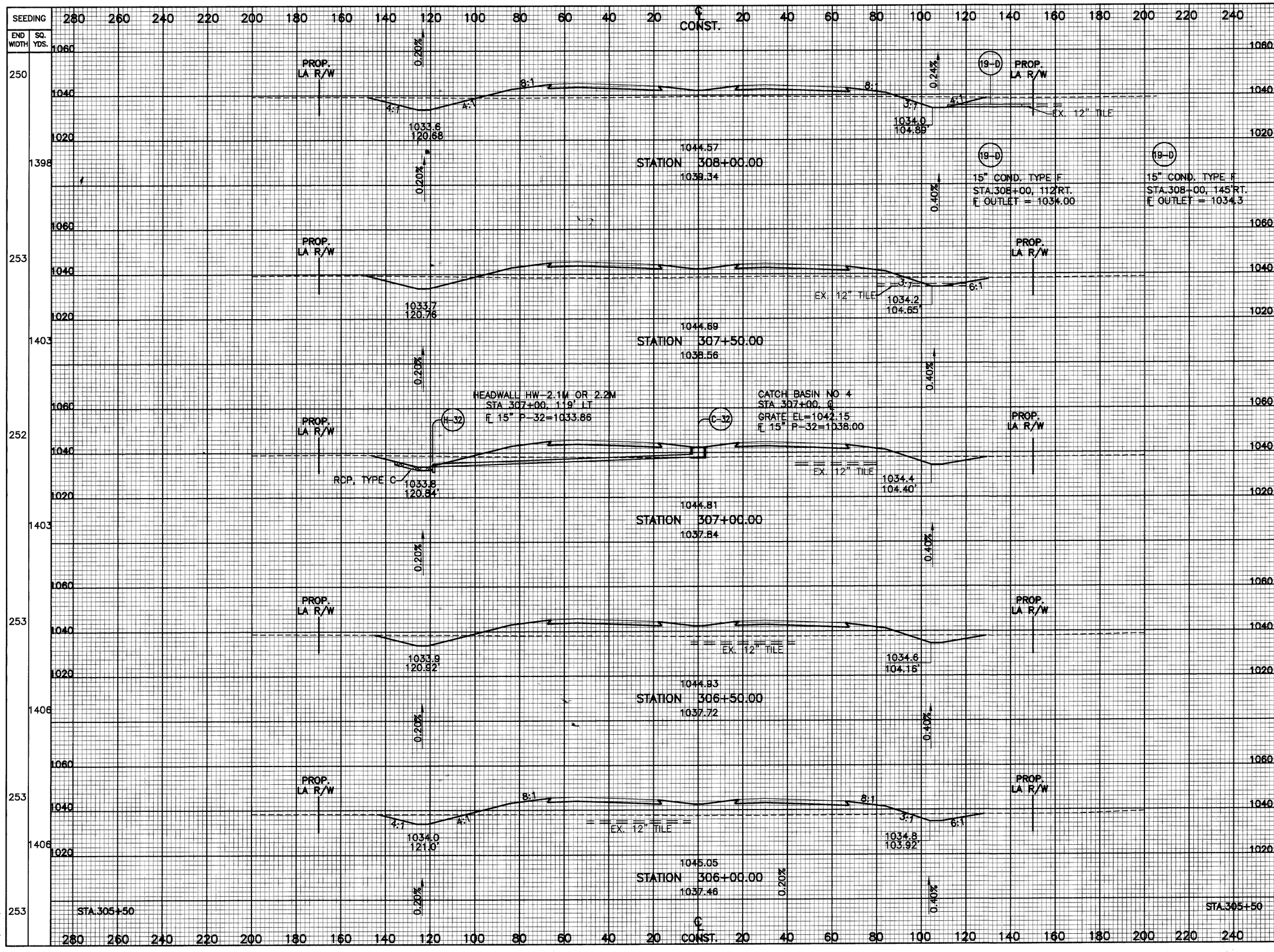
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 303+50 TO STA. 305+50

FAY-35-2.57

129
522

Q:\3590\3590X56



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
250	646		
		458	1313
244	771		
		398	1554
186	907		
		336	1707
177	936		
		305	1794
152	1001		
		262	1896
130	1046		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 306+00 TO STA. 308+00.00

FAY-35-2.57

Q:\3590\3590X57

SEEDING END WIDTH	SEEDING SQ. YDS.
225	1114
176	1212
207	
229	1273
229	1339
253	1398
250	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
276	516	438	987
AHEAD 197			
BACK 386	549		
		621	1054
284	589		
		514	1125
271	625		
		488	1181
256	650		
		469	1200
250	646		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 308+50 TO STA. 310+50.00

FAY-35-2.57

131
522

STA.308+00 — 1044.45 — STATION 308+50.00 — 1039.07
0.20% — 0.20% — PROP. LA R/W — CHANNEL EASE — 8:1 — 4:1 — 4:1 — 1033.5 120.60 — 3:1 — 4:1 — 1033.88 104.89 — 0.24% — 0.24%

STATION 309+00.00 — 1044.33 — 1039.04
0.20% — 0.24% — PROP. LA R/W — 8:1 — 4:1 — 4:1 — 1033.4 120.52 — 3:1 — 4:1 — 1033.76 104.88

STATION 309+50.00 — 1044.22 — 1039.08
0.20% — 0.24% — CHANNEL EASE — PROP. LA R/W — 8:1 — 4:1 — 4:1 — 1033.3 120.48 — 3:1 — 4:1 — 1033.64 104.91

STATION 310+00.00 — 1044.12 — 1039.15
BACK ONLY | BACK & AHEAD
HEADWALL HW-2.2M STA 310+00, 112' LT E OUTLET=1033.21
HEADWALL HW-2.2M STA 310+00, 99' RT E INLET=1033.46
MATCH LINE STA. 5+50 CHANNEL RELOCATION — CHANNEL EASE — PROP. LA R/W — 4:1 NORM — RCP, TYPE B — S-15 — 8:1 — 4:1 — 4:1 — 1033.36 119.52 — 3:1 — 4:1 — 1033.5 105.05

STATION 310+50.00 — 1044.04 — 1039.24
AHEAD BACK — PROP. LA R/W — 0.32% — 8:1 — 4:1 — 4:1 — 1033.36 119.52 — 8:1 — 3:1 — 4:1 — 1034.4 102.1 — 0.40% — PROP. LA R/W

STA.308+00

Q:\3590\3590X58

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
217	1214	112	469	227	840	
220	1237	133	438	281	808	
225	1253	170	434	357	836	
226	1262	215	468	434	887	
228	1259	253	490	490	932	
225		276	516			

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1060 / 1040 / 1020

PROP. LA R/W

H-33 HEADWALL HW-2.1M OR 2.2M STA 313+00, 107' LT E 15" P-33=1034.17

C-33 CATCH BASIN NO 4 STA 313+00, ℄ GRATE EL=1041.19 L 15" P-33=1036.31

RCP, TYPE C

0.32% 0.40% 1.8%

6:1 3:1

1034.16 107.67'

1038.2 90.12'

1043.85 STATION 313+00.00 1038.44

1034.0 108.18'

1038.0 90.75'

1043.86 STATION 312+50.00 1038.76

1033.84 108.72'

1037.1 93.54'

1043.88 STATION 312+00.00 1038.60

1033.68 109.32'

1036.2 96.33'

1043.92 STATION 311+50.00 1039.41

1033.52 109.95'

1035.3 99.18'

1043.97 STATION 311+00.00 1039.38

STA.310+50

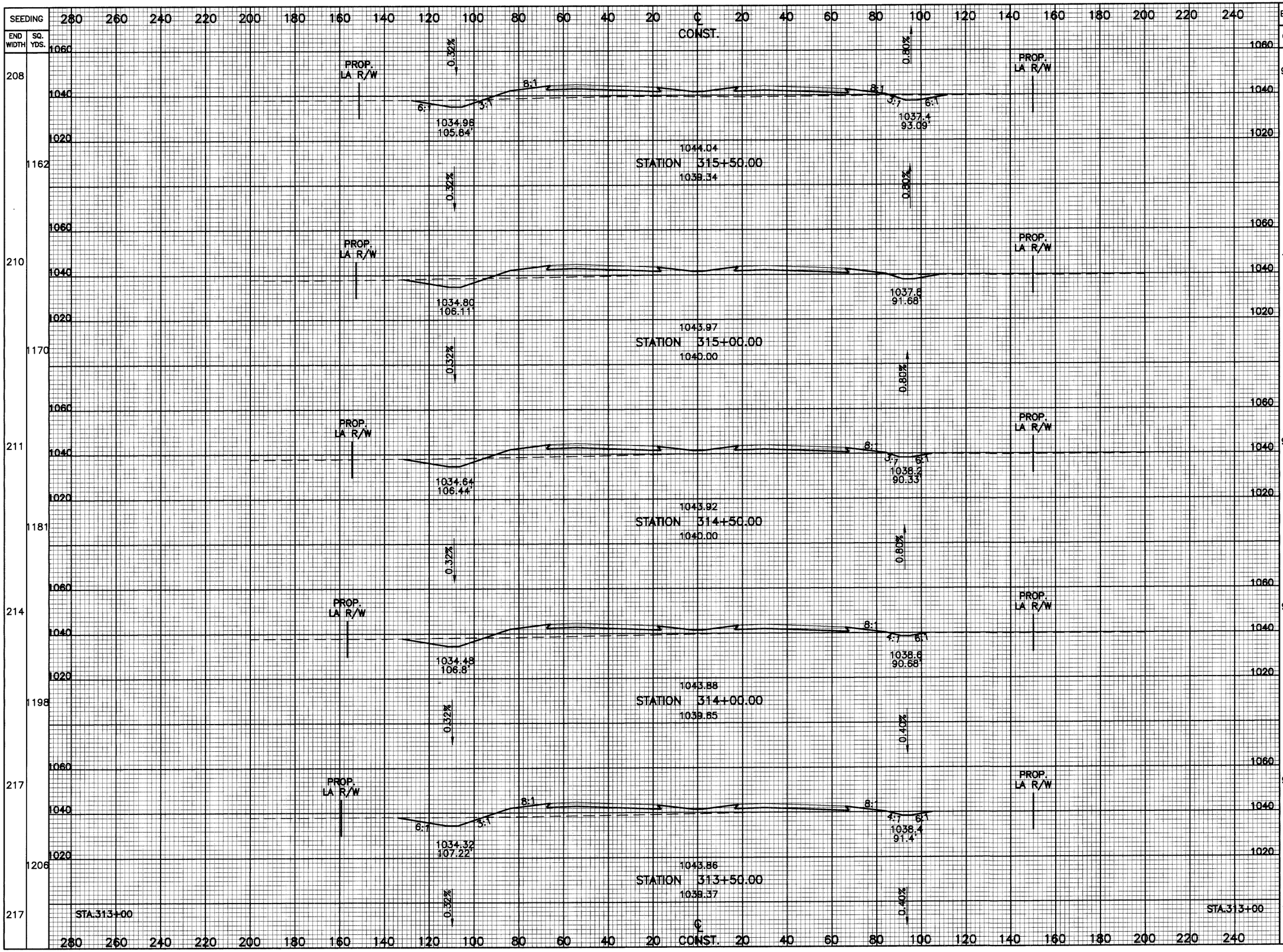
Q:\3590\3590X59

CROSS SECTIONS
STA. 311+00.00 TO STA. 313+00.00

FAY-35-2.57

132 / 522

SEEDING END WIDTH	SEEDING SQ. YDS.
208	1162
210	1170
211	1181
214	1198
217	1206
217	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
90	511		
		182	869
106	427		
		185	800
93	436		
		170	825
90	454		
		175	867
98	482		
		195	881
112	469		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 313+50 TO STA. 315+50.00

FAY-35-2.57

133
522

O:\3590\3590X60

SEEDING END WIDTH	SEEDING SQ. YDS.
206	1148
207	1153
208	1156
208	1156
208	1156
208	

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1060 / 1040 / 1020

PROP. LA R/W

0.28%

6:1 5:1 8:1

1035.76 105.03'

1044.57

STATION 318+00.00

1038.43

0.24%

8:1 5:1 6:1

1036.96 98'

0.32%

1035.60 105.15'

1044.45

STATION 317+50.00

1039.48

0.24%

1036.84 98'

0.32%

1035.44 105.27'

1044.33

STATION 317+00.00

1038.81

0.24%

1036.72 98'

0.32%

HEADWALL HW-2.1M OR 2.2M
STA 316+50, 103' LT
℄ OUTLET=1035.29

RCP, TYPE C-S-16

1035.28 105.42'

1044.22

STATION 316+50.00

1038.92

HEADWALL HW-2.1M OR 2.2M
STA 316+50,93' RT
℄ INLET=1036.58

1036.6 96.03'

0.80%

0.32%

6:1 5:1 8:1

1035.12 105.6'

1044.12

STATION 316+00.00

1038.97

1037.0 94.53'

0.80%

0.32%

STA.315+50

STA.315+50

℄ CONST.

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
113	578	201	1073
104	580	188	1153
99	665	200	1193
116	623	204	1119
104	585	180	1015
90	511		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS

STA. 316+00.00 TO STA. 318+00.00

FAY-35-2.57

134
522

O:\3590\3590X61

SEEDING		END AREA		VOLUME		CALCULATED J.J.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
207	1150	61	747	123	1333	
207	1153	71	692	136	1256	
208	1156	75	664	155	1176	
208	1156	92	606	180	1104	
208	1150	102	586	200	1078	
206		113	578			

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1060 / 1040 / 1020

PROP. LA R/W

0.32%

0.24%

8:1

6:1

3:1

1036.56 104.37'

1037.56 96'

1045.17

STATION 320+50.00

1039.07

1036.4 104.55'

1037.44 96'

1045.05

STATION 320+00.00

1039.47

1036.24 104.67'

1037.32 96'

1044.93

STATION 319+50.00

1039.45

1036.08 104.79'

1037.2 96'

1044.81

STATION 319+00.00

1039.39

1035.92 104.91'

1037.08 96'

1044.69

STATION 318+50.00

1039.36

0.26%

STA.318+00

℄ CONST.

CROSS SECTIONS

STA. 318+50 TO STA. 320+50.00

FAY-35-2.57

135
522

O:\3590\3590X62

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
206	1145	36	771	62	1495
206	1148	31	843	55	1595
207	1150	28	879	58	1623
207	1150	34	873	72	1574
207	1150	43	827	97	1458
207	STA.320+50	61	747		

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ 20 40 60 80 100 120 140 160 180 200 220 240

CONST.

PROP. LA R/W

0.40%

6:1 3:1 8:1

1037.4, 103.71'

1038.2, 95.88'

1045.77

STATION 323+00.00

1039.70

0.32%

1037.2, 103.95'

1038.04, 96'

1045.65

STATION 322+50.00

1039.12

0.24%

1037.04, 104.07'

1037.92, 96'

1045.53

STATION 322+00.00

1038.79

1036.88, 104.19'

1037.8, 96'

1045.41

STATION 321+50.00

1038.73

HEADWALL HW-2.1M OR 2.2M
STA 321+00, 103' LT
℄ 15" P-34=1036.70

(H-34)

CATCH BASIN NO 4
STA 321+00, ℄
GRATE EL=1042.63
℄ 15" P-34=1038.18

(C-34)

RCP, TYPE C

1036.72, 104.31'

1037.68, 96'

1045.29

STATION 321+00.00

1038.86

STA.320+50

CALCULATED J.L.G.
CHECKED W.G.Y.

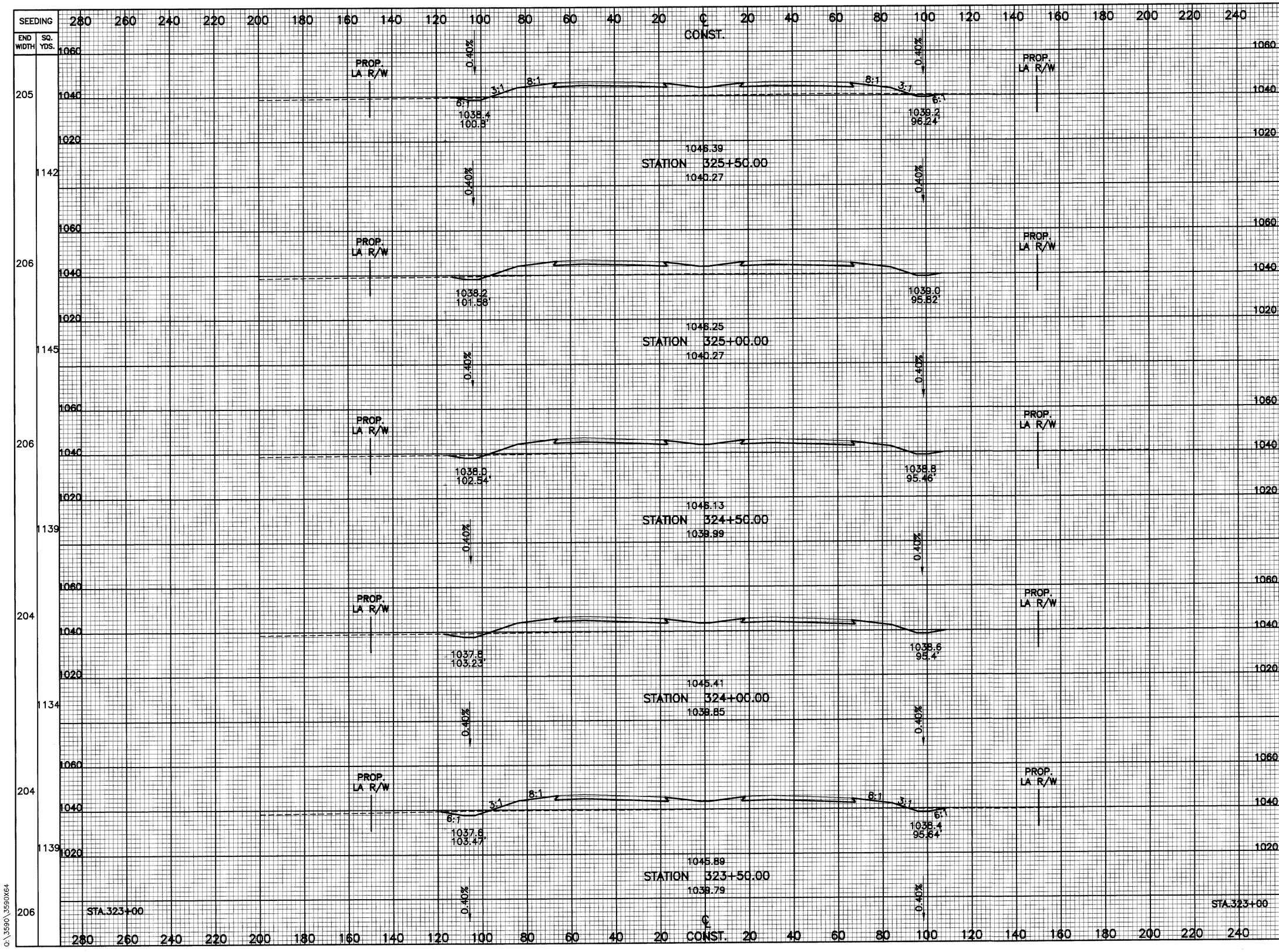
CROSS SECTIONS

STA. 321+00.00 TO STA. 323+00.00

FAY-35-2.57

136
522

Q:\3590\3590X63



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
22	754		
		42	1404
23	762		
		46	1429
26	781		
		56	1439
34	773		
		68	1425
39	766		
		70	1424
36	771		

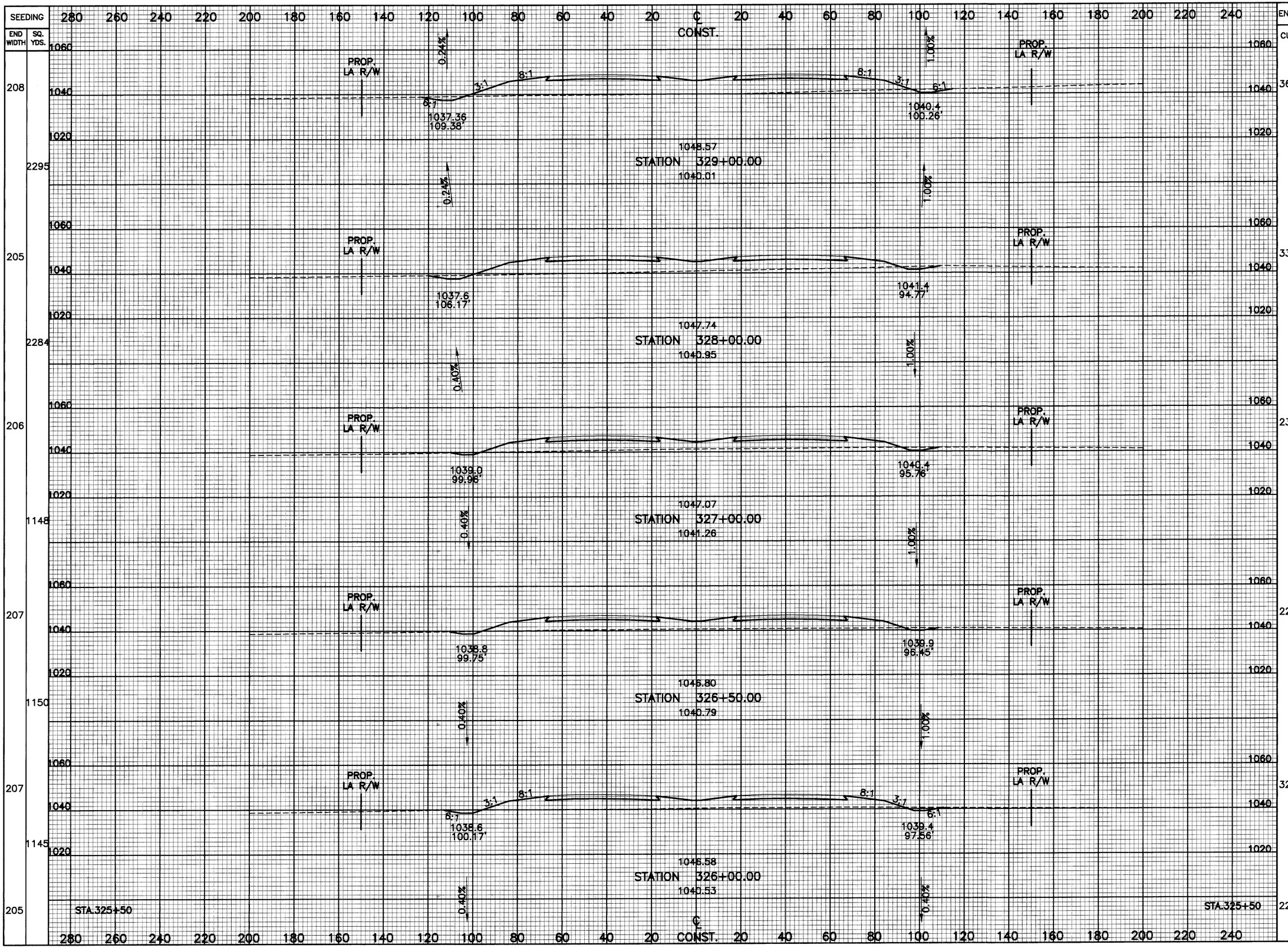
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 323+50 TO STA. 325+50.00

FAY-35-2.57

Q:\3590\3590X64

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
208	2295	36	1199	128	3889	
205	2284	33	901	104	3024	
206	1148	23	732	42	1381	
207	1150	22	759	50	1405	
207	1145	32	758	50	1400	
205		22	754			

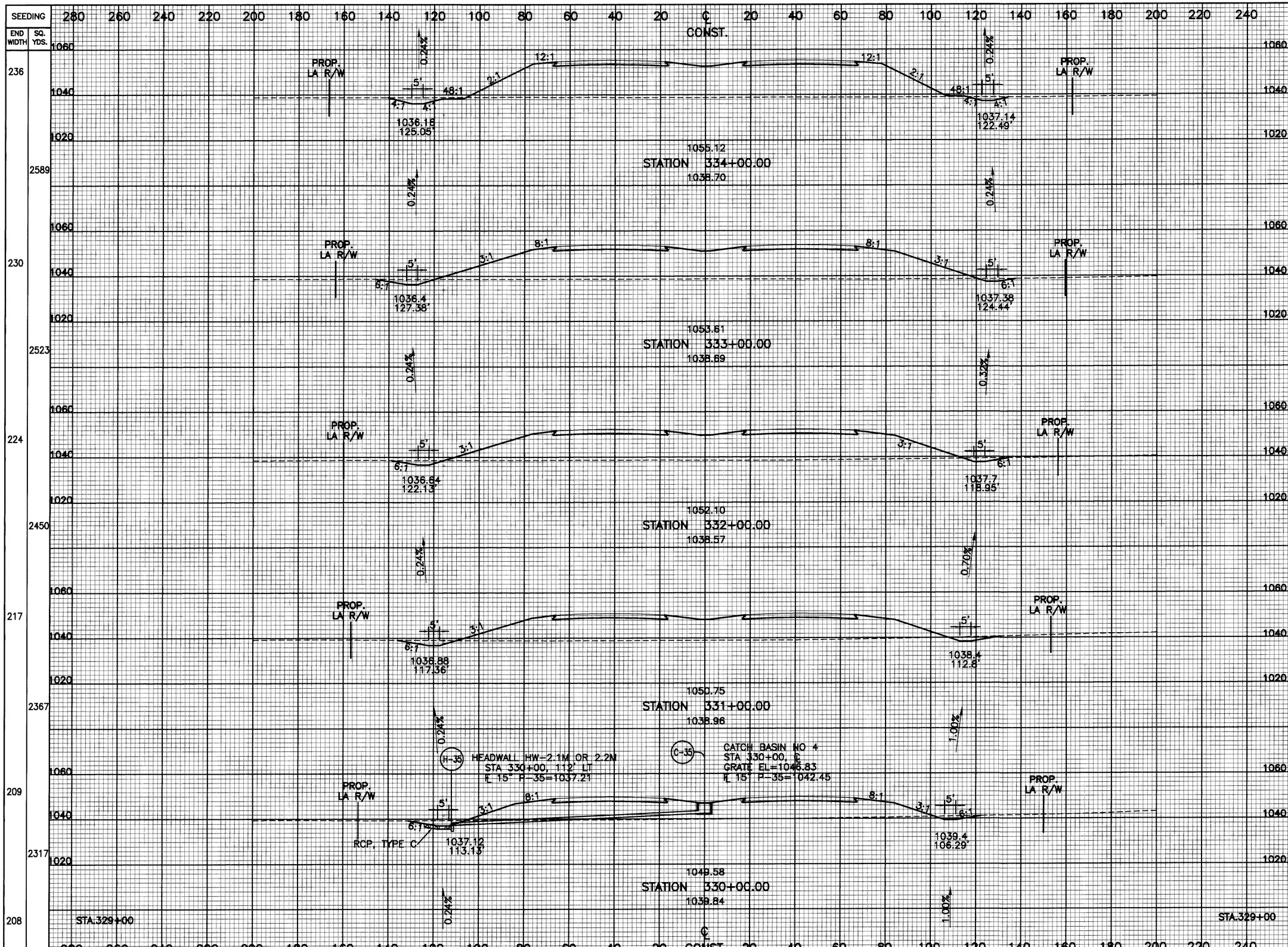


CROSS SECTIONS
STA. 326+00 TO STA. 329+00.00

FAY-35-2.57

138
522

Q:\3590\3590X65



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
67	2693		
		215	9862
49	2632		
		180	9058
48	2259		
		200	7624
60	1858		
		213	6132
55	1453		
		169	4917
36	1199		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 330+00.00 TO STA. 334+00.00

FAY-35-2.57

Q:\3590\3590X66

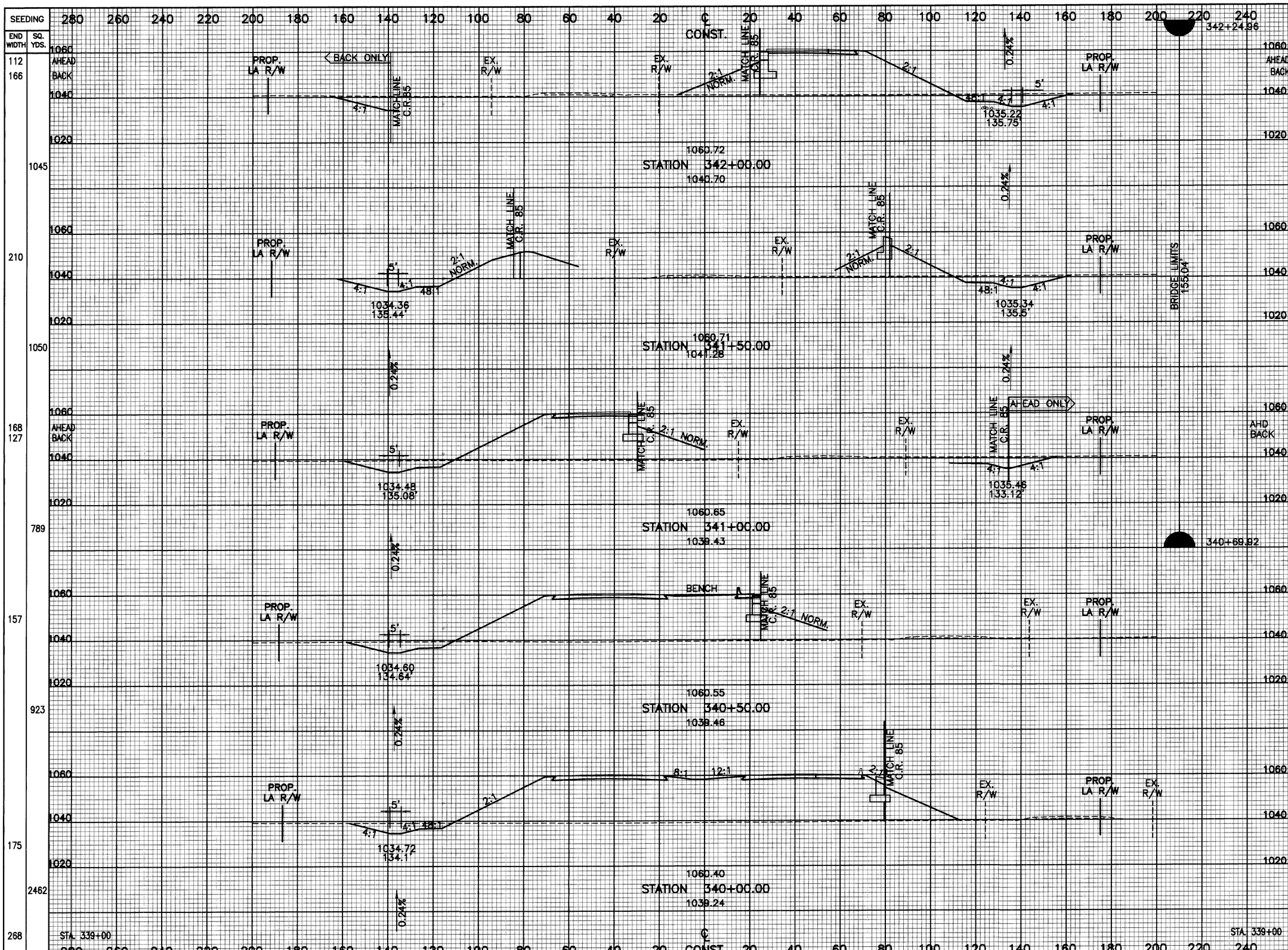
SEEDING		END AREA		VOLUME		CALCULATED
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	J.L.G.
						CHECKED
						W.G.Y.
268		1953	3453			
	2956			719	12330	
264		1933	3205			
	2917			586	11769	
261		1233	3150			
	2850			412	11443	
252		99	3029			
	2756			297	10847	
244		61	2828			
	2667			237	10224	
236		67	2693			

CROSS SECTIONS
STA. 335+00 TO STA. 339+00.00

FAY-35-2.57

140
522

O:\3590\3590X67



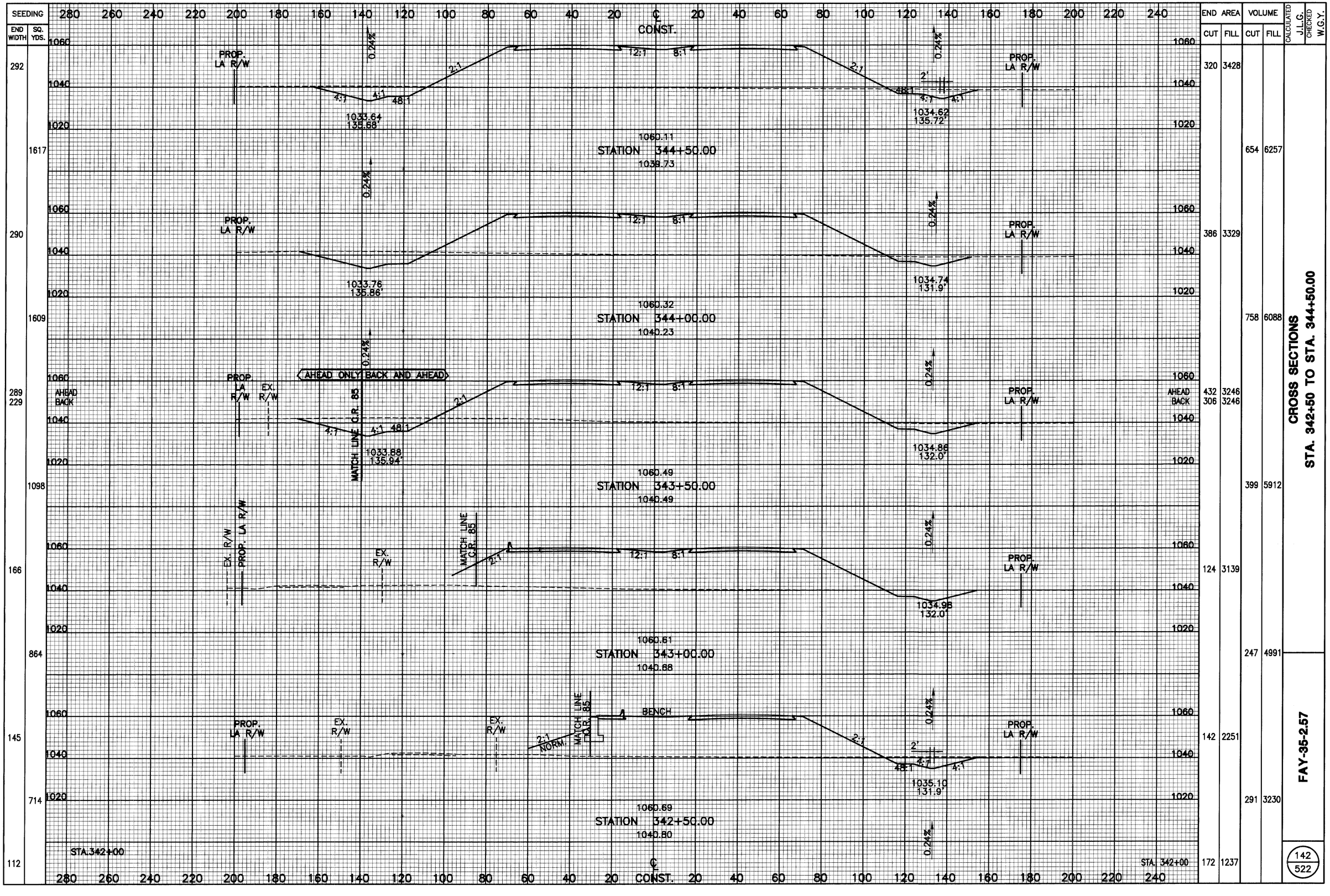
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
172	1237		
253	1237		
		546	1504
336	387		
		494	1477
197	1208		
142	1208		
		254	3239
132	2290		
		234	5195
120	3320		
		584	12543
195	3453		

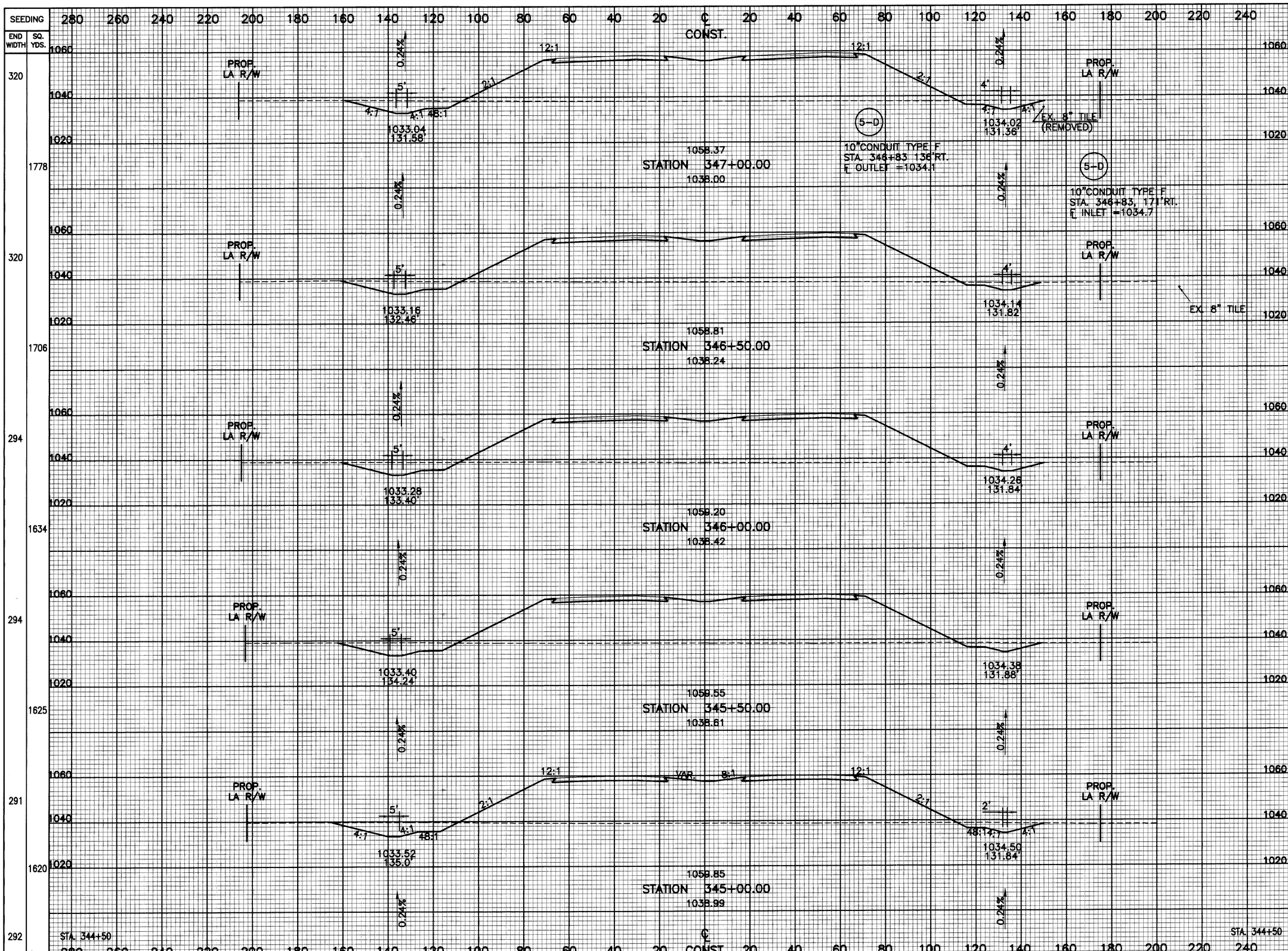
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 340+00.00 TO STA. 342+00.00

FAY-35-2.57

Q:\3590\3590X68


SEEDING
END WIDTH
SQ. YDS.
280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ 20 40 60 80 100 120 140 160 180 200 220 240
CONST.
1060
1040
1020
PROP. LA R/W
EX. R/W
0.24%
2:1
12:1
8:1
4:1
48:1
2'
1033.64
135.68'
1060.11
STATION 344+50.00
1038.73
1034.62
135.72'
292
1617
1033.76
135.86'
1060.32
STATION 344+00.00
1040.23
1034.74
131.9'
290
1609
AHEAD ONLY BACK AND AHEAD
AHEAD
BACK
MATCH LINE C.R. 85
1033.88
135.94'
1060.49
STATION 343+50.00
1040.49
1034.86
132.0'
289
229
1098
EX. R/W
PROP. LA R/W
1060.61
STATION 343+00.00
1040.68
1034.98
132.0'
166
864
BENCH
2:1 NORM.
1035.10
131.9'
145
714
1060.69
STATION 342+50.00
1040.80
STA. 342+00
112
END AREA
CUT
FILL
VOLUME
CALCULATED J.L.G.
CHECKED W.G.Y.
320 3428
654 6257
386 3329
758 6088
AHEAD 432 3246
BACK 306 3246
399 5912
124 3139
247 4991
142 2251
291 3230
172 1237
CROSS SECTIONS
STA. 342+50 TO STA. 344+50.00
FAY-35-2.57
142
522
O:\3590\3590X69



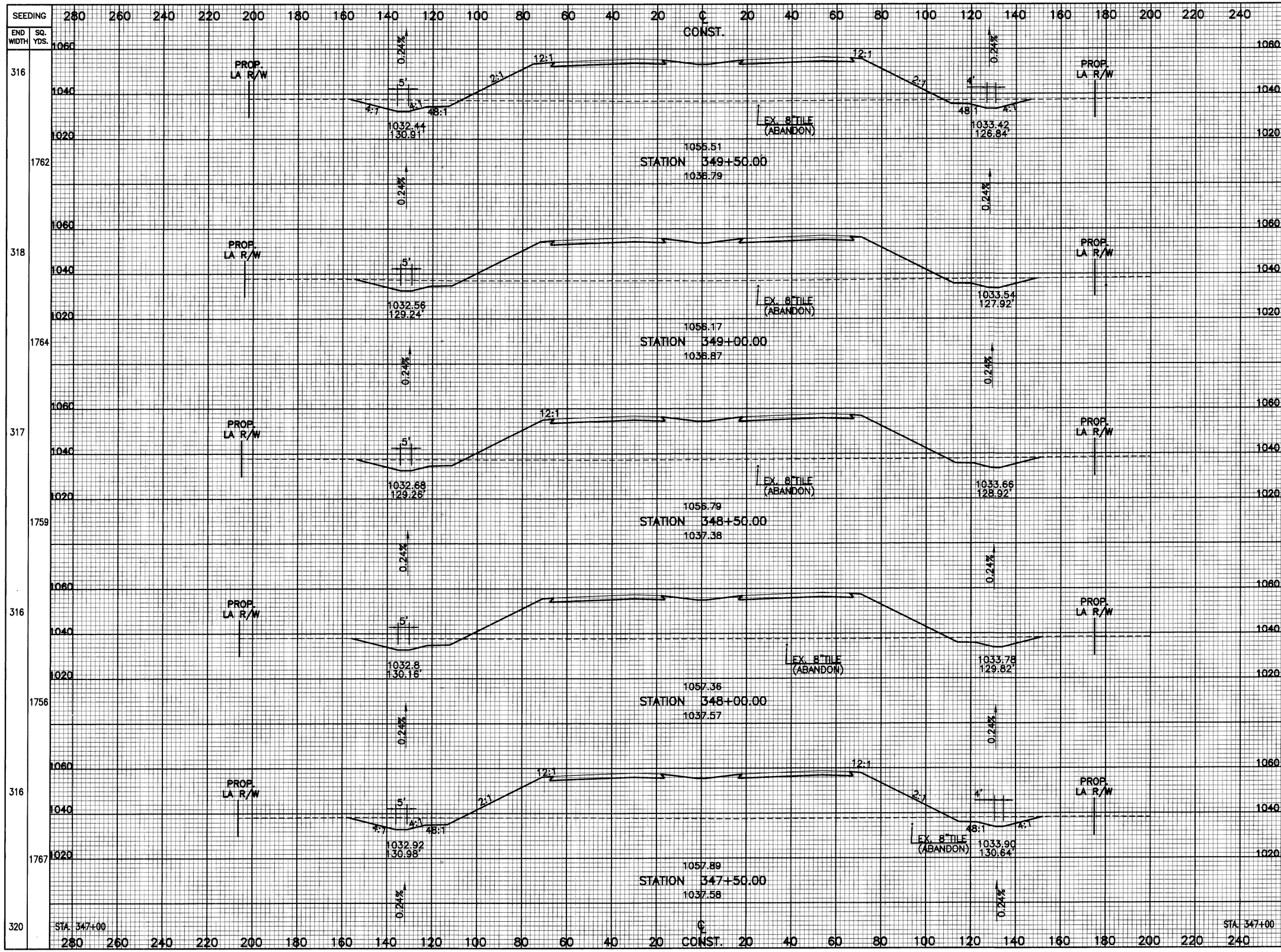
SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
320		251	3394		
	1778			454	6337
320		239	3449		
	1706			452	6413
294		249	3477		
	1634			474	6477
294		262	3518		
	1625			518	6498
291		297	3499		
	1620			572	6414
292		320	3428		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 345+00 TO STA. 347+00.00

FAY-35-2.57

Q:\3590\3590X70



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME
227	3072		
		422	5822
228	3215		
		431	5920
237	3178		
		448	5963
246	3262		
		465	6117
256	3344		
		470	6239
251	3394		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 347+50 TO STA. 349+50.00

FAY-35-2.57

Q:\3590\3590X71

SEEDING END WIDTH	SEEDING SQ. YDS.
308	1717
310	1723
310	1723
310	1731
313	1748
316	

HEADWALL HW-2.1M
STA. 352+00, 131.6' LT
℄ 15" P-36 = 1031.85

CATCH BASIN NO.4
STA. 352+00, 6' RT
GRATE EL. =1048.86
℄ 15" P-36=1043.54

CONST. ℄

PROP. LA R/W

TEMP. R/W

RCP TYPE C

EX. 10" TILE (ABANDON)

EX. 8" TILE (ABANDON)

6-D

0.24%

6:1 3:1 8:1 12:1 2:1 4:1 48:1

1031.84 132.21'

1032.82 125.58'

1051.52
STATION 352+00.00
1036.86

1031.96 134.52'

1032.94 121.8'

1052.41
STATION 351+50.00
1035.99

1032.08 136.68'

1033.06 123.04'

1053.25
STATION 351+00.00
1036.49

1032.20 140.97'

1033.18 124.4'

1054.05
STATION 350+50.00
1036.76

1032.32 138.94'

1033.3 125.66'

1054.80
STATION 350+00.00
1036.70

STA.349+50

STA. 349+50

℄ CONST.

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
180	2664	352	4987
200	2721	368	5138
197	2827	374	5374
206	2976	391	5600
216	3071	411	5688
227	3072		

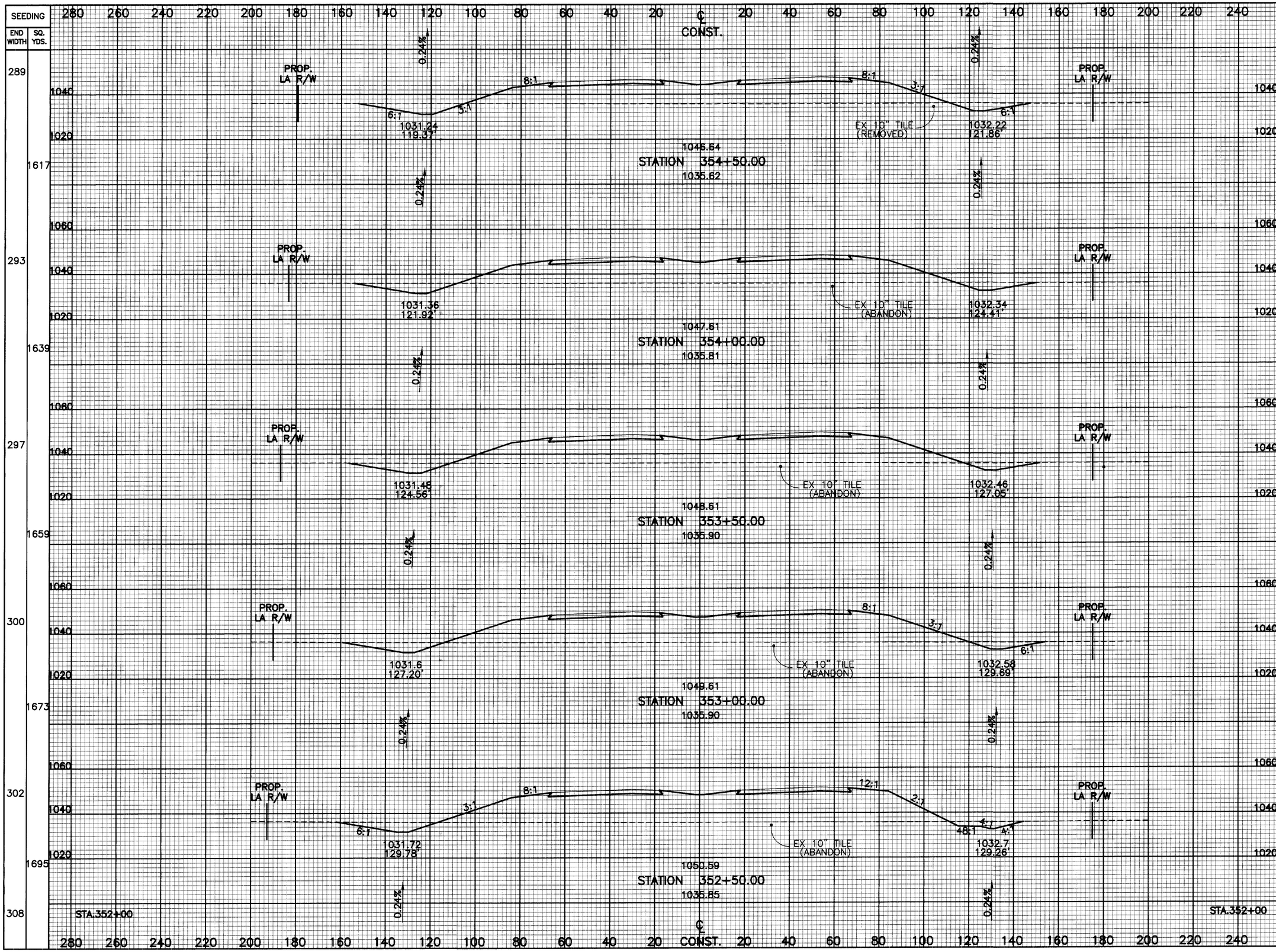
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 350+00.00 TO STA. 352+00.00

FAY-35-2.57

145
522

O:\3590\3590X72



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
194	1701		
		354	3355
188	1922		
		334	3769
172	2148		
		316	4189
169	2376		
		315	4559
171	2547		
		325	4825
180	2664		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 352+50 TO STA. 354+50.00

FAY-35-2.57

Q:\3590\3590X73

SEEDING	
END WIDTH	SQ. YDS.
258	1428
256	1423
256	1492
281	1570
284	1592
289	

END AREA		VOLUME		CALCULATED J.L.G.	CHECKED W.G.Y.
CUT	FILL	CUT	FILL		
247	915				
		443	1837		
231	1069				
		408	2117		
209	1217				
		374	2384		
194	1357				
		365	2675		
200	1532				
		365	2994		
194	1701				

CROSS SECTIONS
STA. 355+00 TO STA. 357+00.00

FAY-35-2.57

147
522

0:\3590\3590X74

SEEDING	
END WIDTH	SQ. YDS.
258	
	1434
258	
	1434
258	
	1434
258	
	1434
258	

END AREA		VOLUME		CALCULATED
CUT	FILL	CUT	FILL	J.L.G.
383	247			CHECKED
		652	556	W.G.Y.
321	353			
		573	787	
297	497			
		538	1042	
284	628			
		509	1289	
265	764			
		474	1555	
247	915			

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

PROP. LA R/W

0.24%

6:1 3:1 8:1

1030.04 102.87'

1039.84

STATION 359+50.00

1038.78

0.36%

8:1 3:1 6:1

1030.78 106.08'

1040.36

STATION 359+00.00

1036.62

1030.16 103.77'

1039.96 106.80'

1040.84

STATION 358+50.00

1036.21

1030.28 104.85'

1031.14 107.70'

1041.37

STATION 358+00.00

1035.92

1030.4 106.08'

1031.32 108.75'

1041.96

STATION 357+50.00

1035.75

1030.52 107.49'

1031.50 109.98'

1.20%

STA.357+00

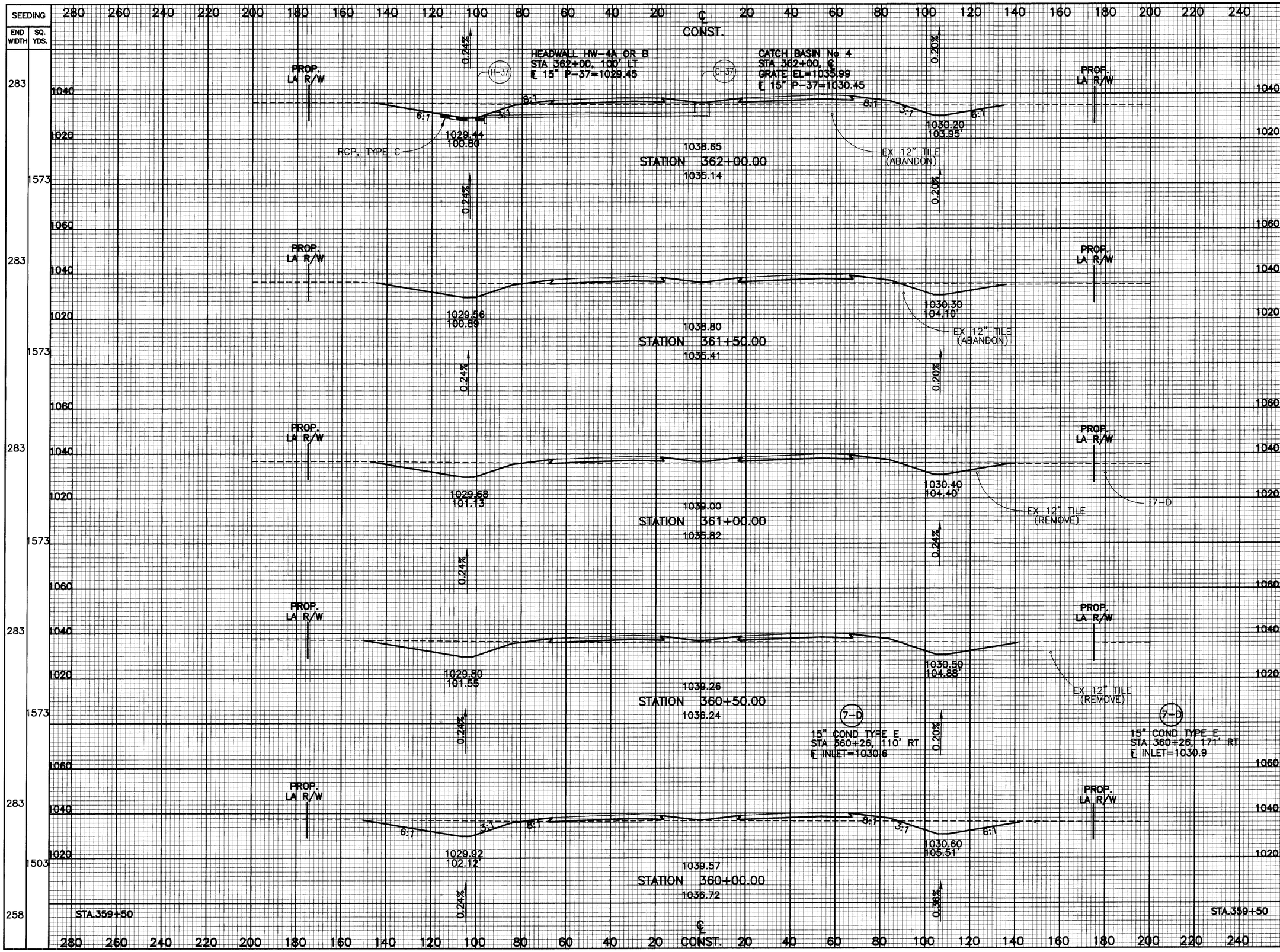
STA.357+00

CROSS SECTIONS STA. 357+50 TO STA. 359+50.00

FAY-35-2.57

148
522

Q:\3590\3590X75



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
318	289		
		605	510
335	261		
		651	472
368	248		
		715	425
404	211		
		756	386
412	205		
		737	419
383	247		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 360+00.00 TO STA. 362+00.00

FAY-35-2.57

Q:\3590\3590X76

CROSS SECTIONS
STA. 362+50 TO STA. 364+50.00

CALCULATED J.L.G. CHECKED W.G.Y.

FAY-35-2.57

150
522

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
281	1564	341	257	625	510
282	1570	334	293	633	503
283	1573	349	250	633	475
283	1573	334	263	601	527
283	1573	315	306	587	551
283		318	289		

STATION 364+50.00
1034.70
1028.84 100.80
1038.05
1029.70 103.65

STATION 364+00.00
1034.51
1028.96 100.80
1038.17
1029.80 103.71

STATION 363+50.00
1034.92
1029.08 100.80
1038.29
1029.90 103.37

STATION 363+00.00
1035.07
1029.20 100.80
1038.41
1030.00 103.83

STATION 362+50.00
1034.97
1029.32 100.80
1038.53
1030.10 103.89

PROP. LA R/W
EX. 12" TILE (ABANDON)
0.24%
0.20%
6:1
3:1
8:1
STA.362+00
CONST.
1040
1020
1060

0:\3590\3590X77

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
281		311	276		
	1567			599	501
283		335	265		
	1567			620	491
281		334	265		
	1559			612	492
280		327	266		
	1556			613	471
280		335	242		
	1559			626	462
281		341	257		

CALCULATED J.L.G.
CHECKED W.G.Y.

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

PROP. LA R/W

0.24%

0.20%

6:1 3:1 8:1

EX 12" TILE (ABANDON)

1028.24 100.80 — 1037.45 — 1029.20 103.35

STATION 367+00.00

1034.01

1028.36 100.80 — 1037.57 — 1029.20 103.41

STATION 366+50.00

1034.11

1028.48 100.80 — 1037.69 — 1029.40 103.47

STATION 366+00.00

1034.29

1028.60 100.80 — 1037.81 — 1029.50 103.53

STATION 365+50.00

1034.44

1028.72 100.80 — 1037.93 — 1029.60 103.59

STATION 365+00.00

1034.68

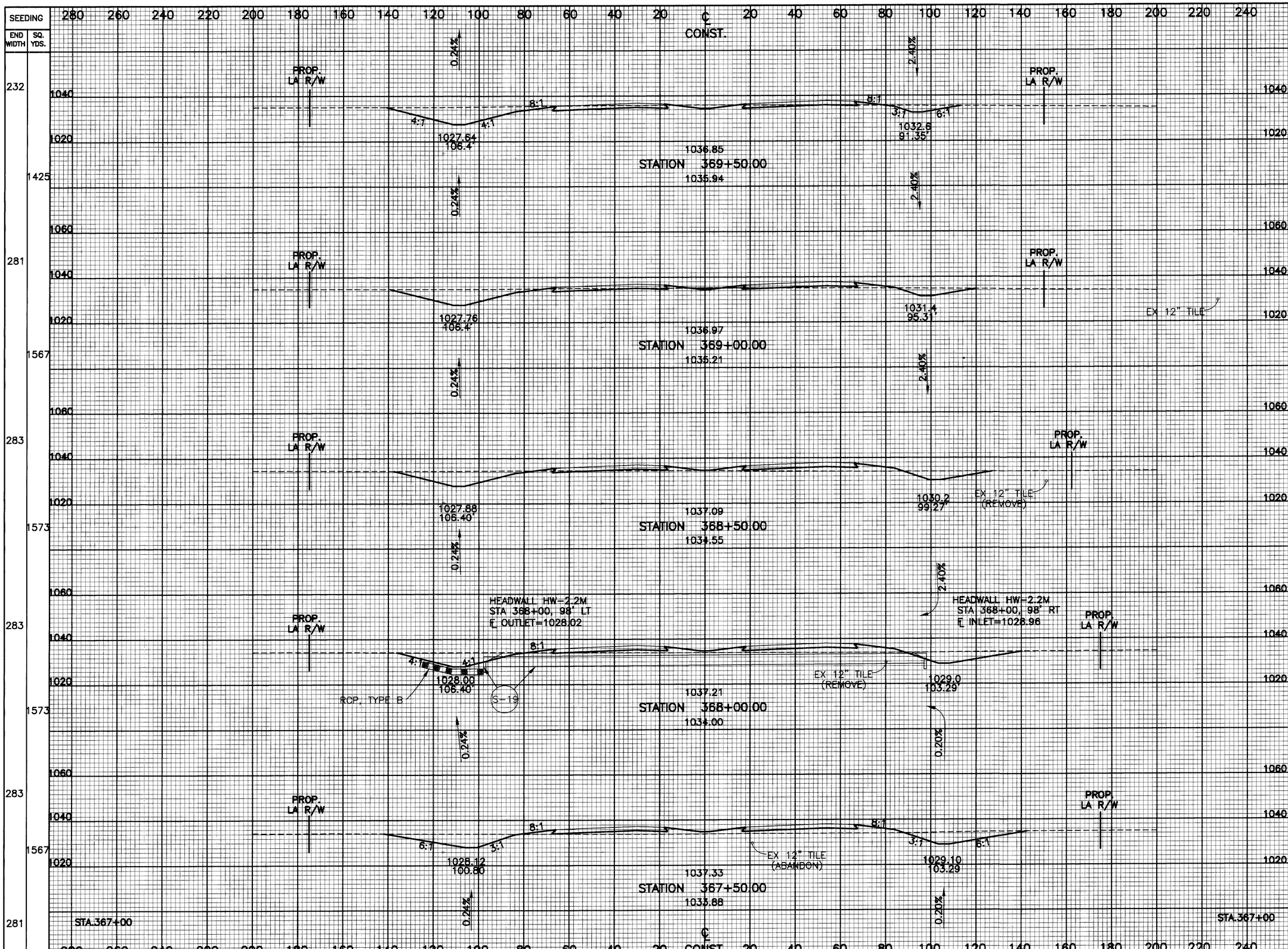
STA.364+50

CROSS SECTIONS
STA. 365+00 TO STA. 367+00.00

FAY-35-2.57

151
522

Q:\3590\3590X78



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
414	15		
		692	71
333	61		
		582	211
295	166		
		579	374
330	237		
		624	478
343	279		
		606	514
311	276		

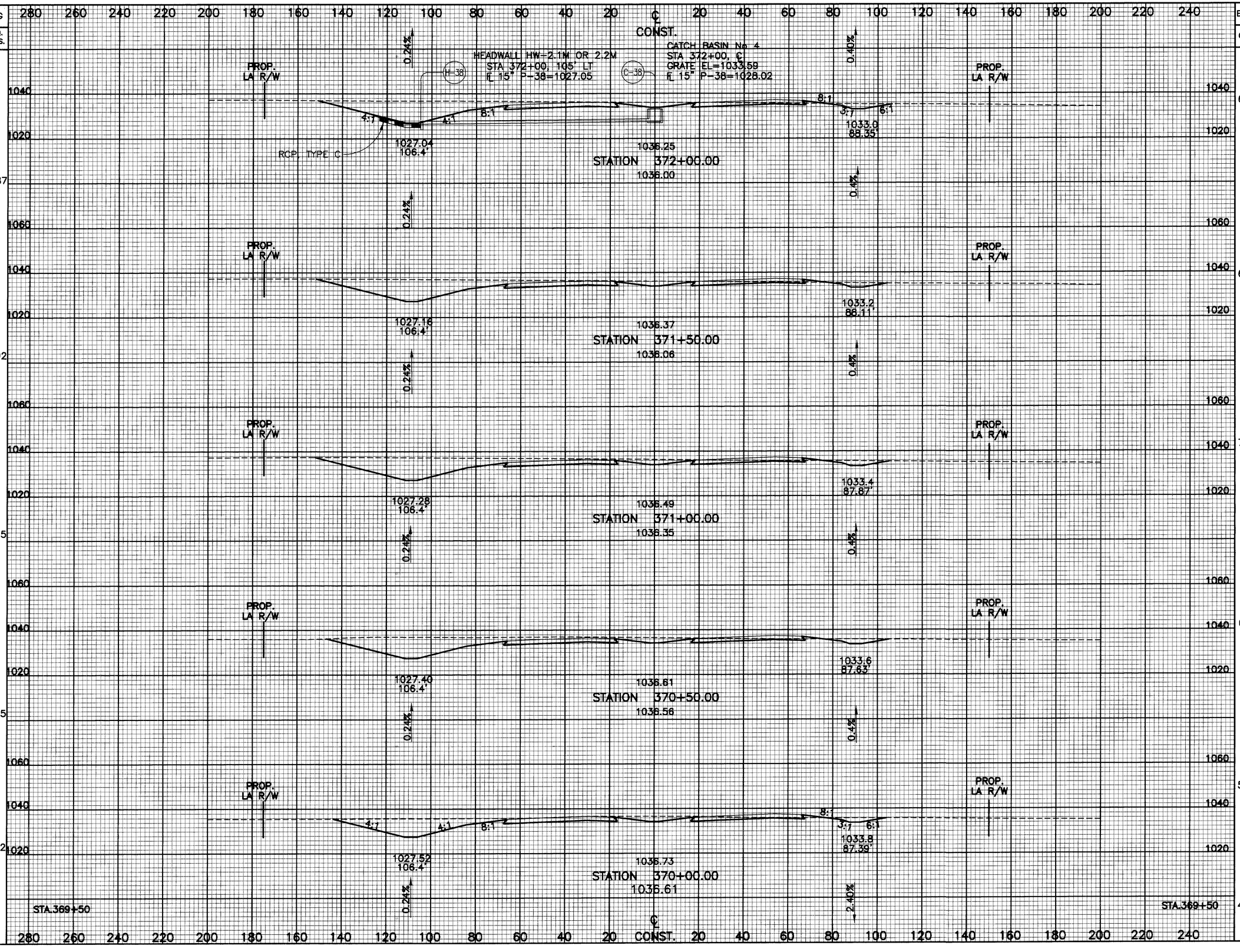
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 367+50 TO STA. 369+50.00

FAY-35-2.57

Q:\3590\3590X79

SEEDING END WIDTH	SEEDING SQ. YDS.
231	1287
232	1292
233	1295
233	1295
233	1292
232	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
698	6		
		1290	13
695	7		
		1323	13
733	6		
		1319	12
691	6		
		1146	13
546	7		
		889	12
414	15		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 370+00.00 TO STA. 372+00.00

FAY-35-2.57

153
522

O:\3590\3590X80

SEEDING END WIDTH	SEEDING SQ. YDS.
232	1289
232	1289
232	1295
234	1300
234	1292
231	STA.372+00

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
478	42	952	64
550	27	1072	43
607	19	1160	27
645	10	1234	16
687	7	1283	13
698	6		

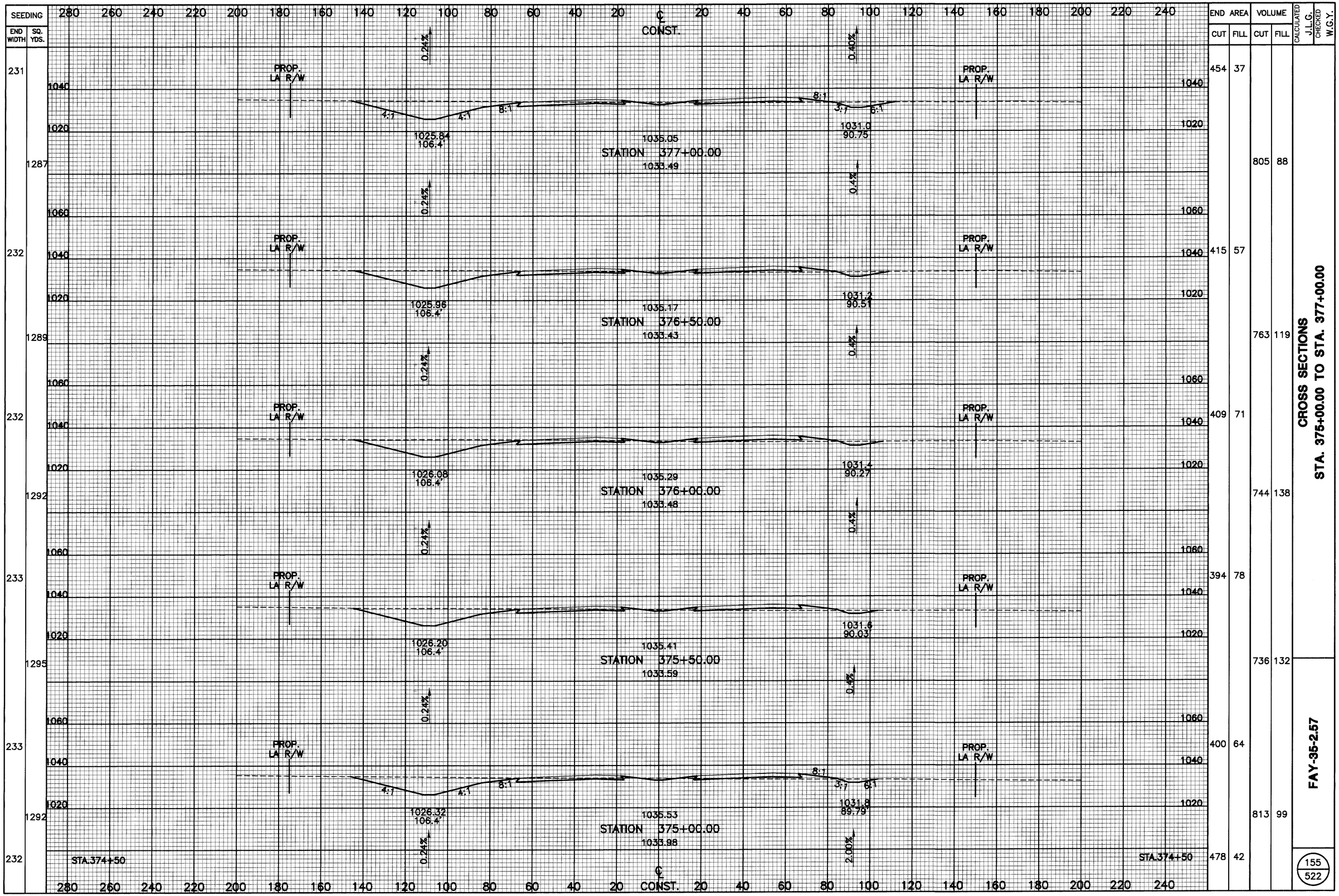
CALCULATED J.L.G.
CHECKED W.G.Y.

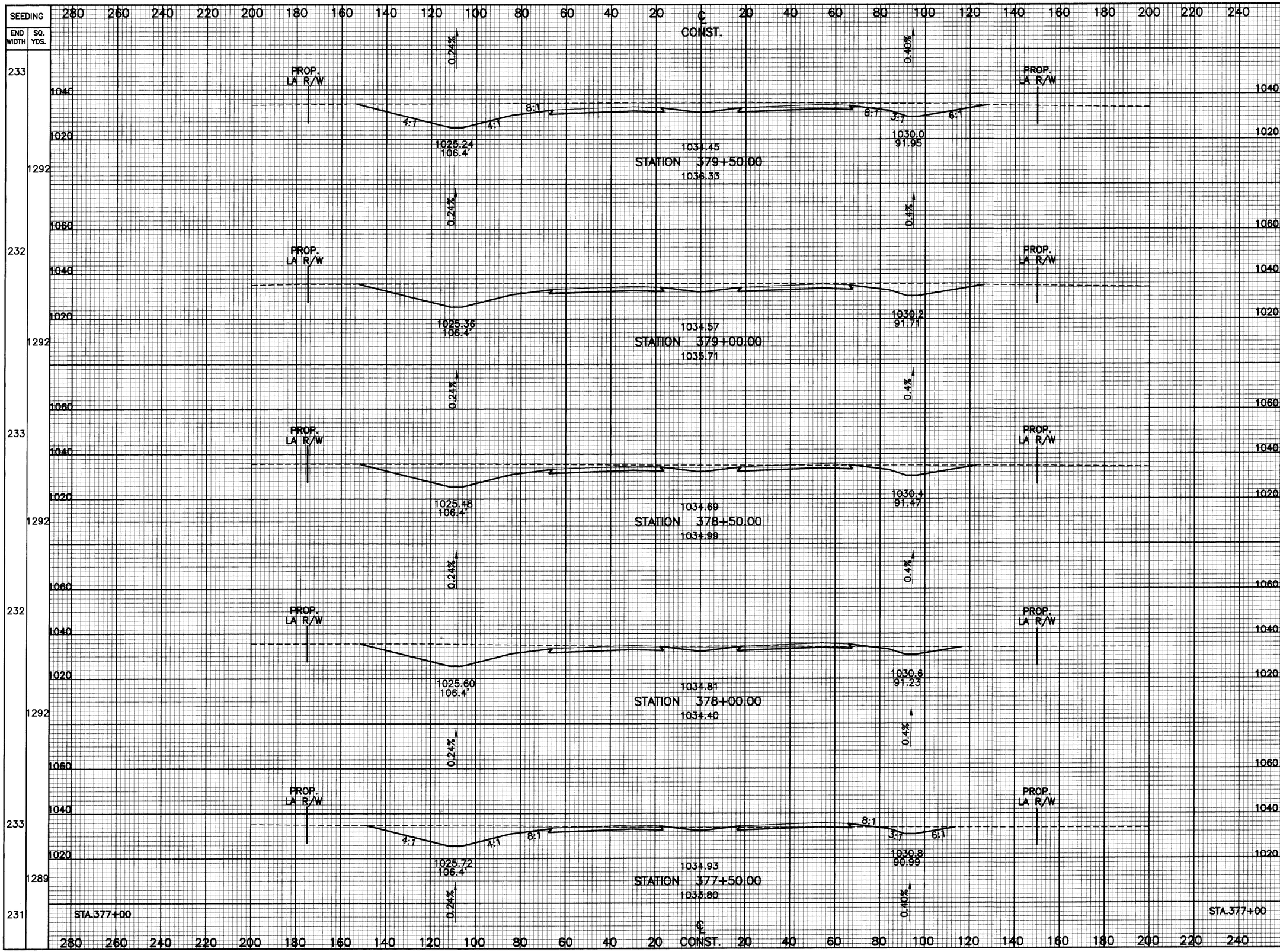
CROSS SECTIONS
STA. 372+50.00 TO STA. 374+50.00

FAY-35-2.57

154
522

Q:\3590\3590X81


SEEDING
END WIDTH
SQ. YDS.
280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240
231
1287
232
1289
232
1292
233
1295
233
1292
232
1040
1020
1060
0.24%
0.40%
0.4%
PROP. LA R/W
4:1
8:1
3:1
6:1
1025.84 106.4'
1035.05
STATION 377+00.00
1033.49
1031.0 90.75
1025.96 106.4'
1035.17
STATION 376+50.00
1033.43
1031.2 90.51
1026.08 106.4'
1035.29
STATION 376+00.00
1033.48
1031.4 90.27
1026.20 106.4'
1035.41
STATION 375+50.00
1033.59
1031.6 90.03
1026.32 106.4'
1035.53
STATION 375+00.00
1033.98
1031.8 89.79'
2.00%
STA.374+50
END AREA
CUT
FILL
454 37
415 57
409 71
394 78
400 64
478 42
VOLUME
CUT
FILL
805 88
763 119
744 138
736 132
813 99
CALCULATED
J.L.G.
CHECKED
W.G.Y.
CROSS SECTIONS
STA. 375+00.00 TO STA. 377+00.00
FAY-35-2.57
155
522
Q:\3590\3590X82



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
1187	0		
		2090	0
1070	0		
		1811	1
885	1		
		1501	7
736	6		
		1207	18
567	13		
		946	47
454	37		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 377+50.00 TO STA. 379+50.00

FAY-35-2.57

Q:\3590\3590X83

SEEDING END WIDTH	SEEDING SQ. YDS.
233	1295
233	1292
232	1289
232	1287
231	1289
233	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
1372	0	2604	0
1440	0	2635	0
1405	0	2514	0
1310	0	2388	0
1268	0	2274	0
1187	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

280 260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

HEADWALL HW-2.1M OR 2.2M
STA 382+25, 108' LT
FL 15" P-39=1024.65
H-39

CATCH BASIN No. 4
STA 382+25, ℄
GRATE EL=1031.04
FL 15" P-39=1026.38
C-39

PROP. LA R/W

RCP, TYPE C

4:1 4:1 8:1 8:1 3:1 6:1

0.24% 0.40%

1024.64 109.44' 1033.85 STATION 382+00.00 1036.70 1029.00 90.60'

1024.76 108.64' 1033.97 STATION 381+50.00 1037.03 1029.20 90.96'

1024.88 107.84' 1034.09 STATION 381+00.00 1036.68 1029.40 91.32'

1025.00 107.04' 1034.21 STATION 380+50.00 1036.73 1029.60 91.71'

1025.12 106.4' 1034.33 STATION 380+00.00 1036.70 1029.80 92.19'

STA.379+50

1040 1020 1060

CROSS SECTIONS
STA. 380+00.00 TO STA. 382+00.00

FAY-35-2.57

157
522

Q:\3590\3590X84

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
230	2562	386+00.00	1203	0	4454	0
231	2578	385+00.00	1202	0	4380	0
233	2595	384+00.00	1163	0	4506	0
234	1298	383+00.00	1270	0	2405	0
233	1295	382+50.00	1327	0	2500	0
233		STA.382+00	1372	0		

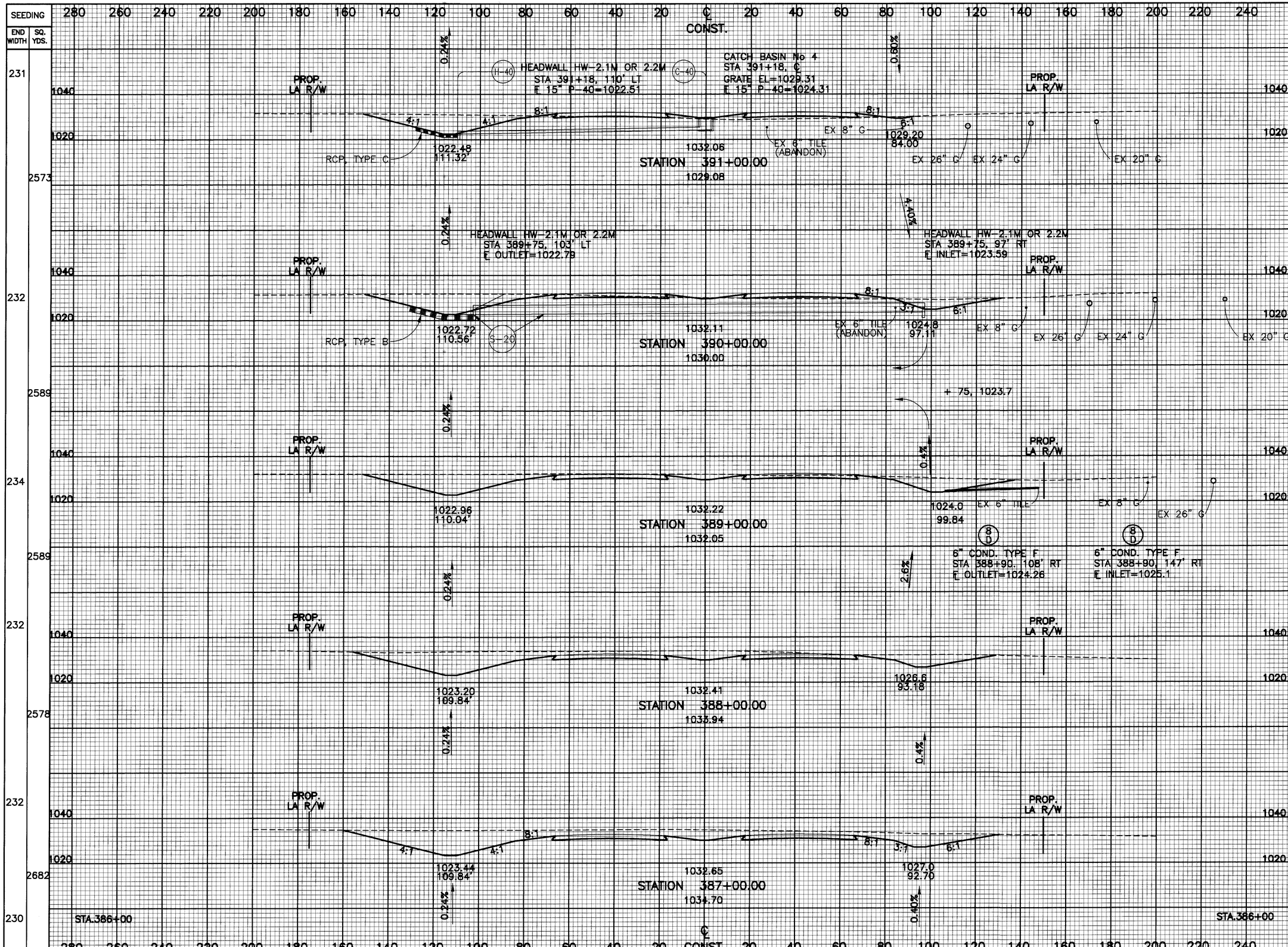
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 382+50.00 TO STA. 386+00.00

FAY-35-2.57

158
522

Q:\3590\3590X85



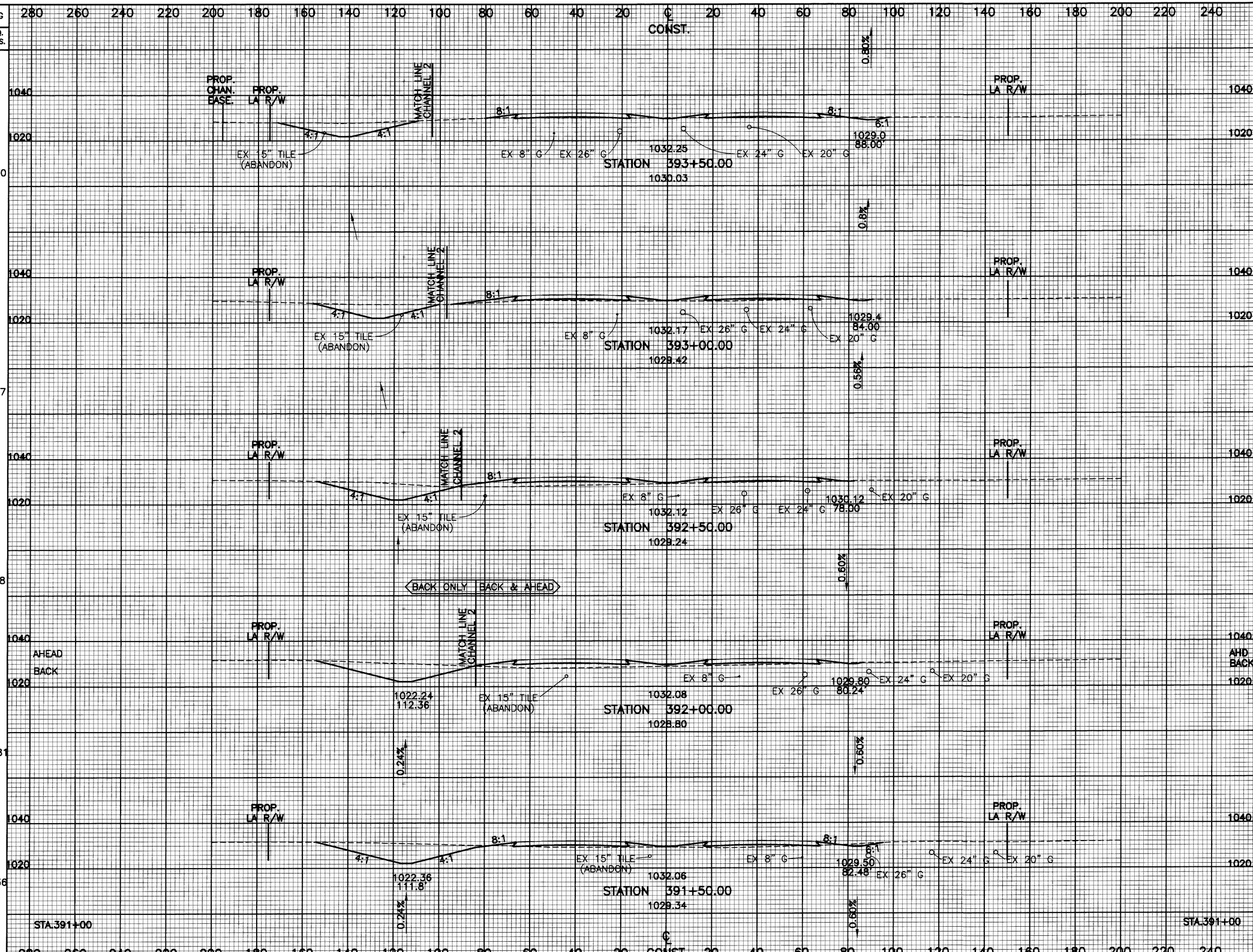
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
348	116		
		1573	343
501	69		
		2417	128
804	0		
		3424	0
1045	0		
		4299	0
1276	0		
		4591	0
1203	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 387+00.00 TO STA. 391+00.00

FAY-35-2.57

Q:\3590\3590X86



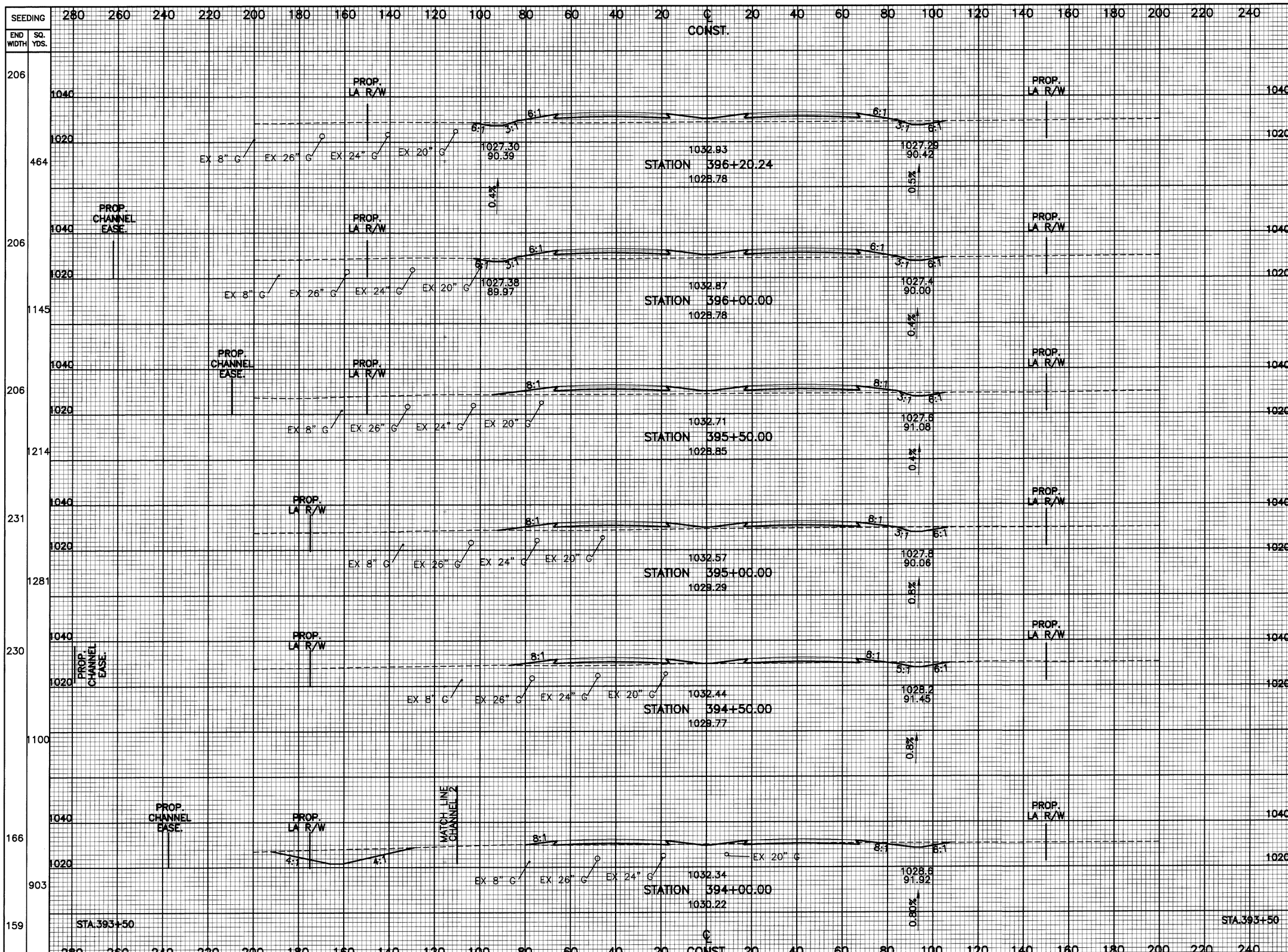
SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
159		18	59		
	870			22	183
154		5	138		
	837			8	312
147		3	198		
	798			7	386
140		4	218		
232		295	218		
	1281			600	323
229		352	130		
	1336			649	228
231		348	116		

CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 391+50.00 TO STA. 393+50.00

FAY-35-2.57

Q:\3590\3590X87



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
35	391		
		26	288
33	375		
		48	675
18	353		
		41	573
26	265		
		55	396
33	162		
		77	207
50	61		
		63	112
18	59		

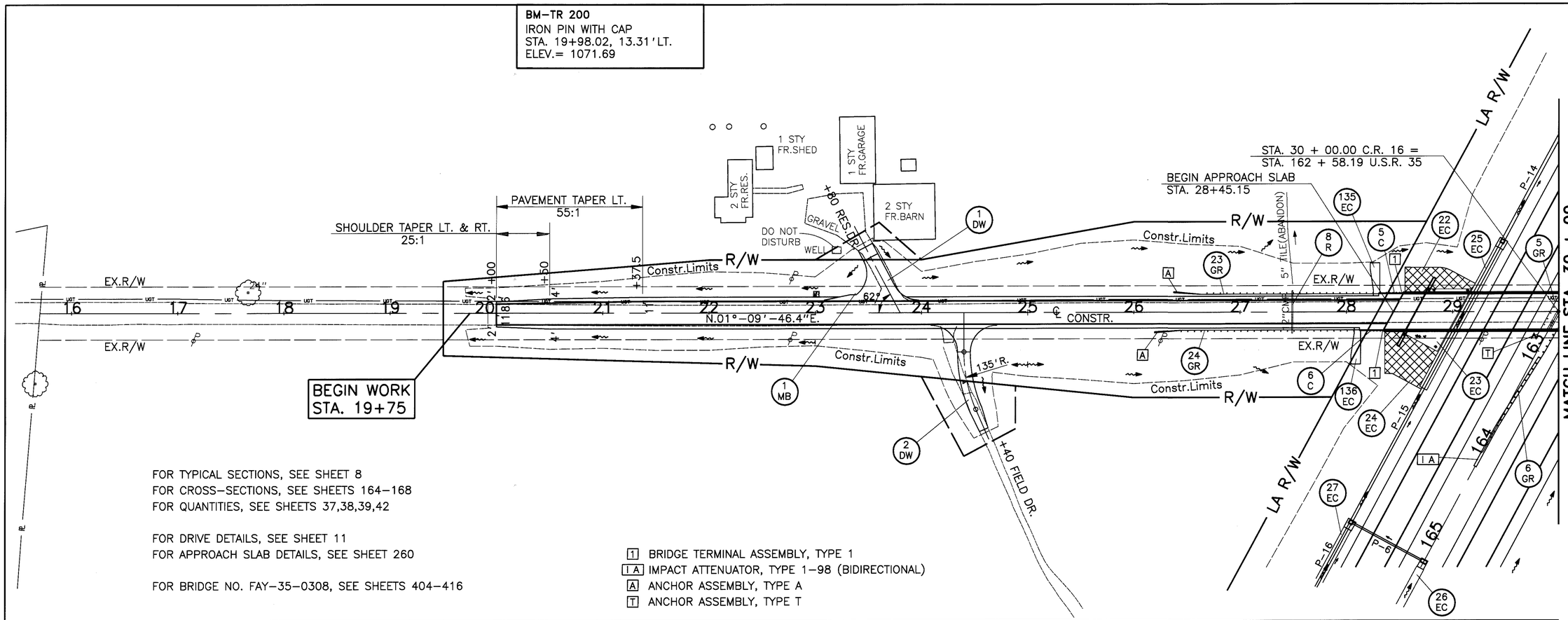
CALCULATED J.L.G.
CHECKED W.G.Y.

CROSS SECTIONS
STA. 394+00.00 TO STA. 396+20.24

FAY-35-2.57

Q:\3590\3590X88

BM–TR 200
IRON PIN WITH CAP
STA. 19+98.02, 13.31' LT.
ELEV.= 1071.69



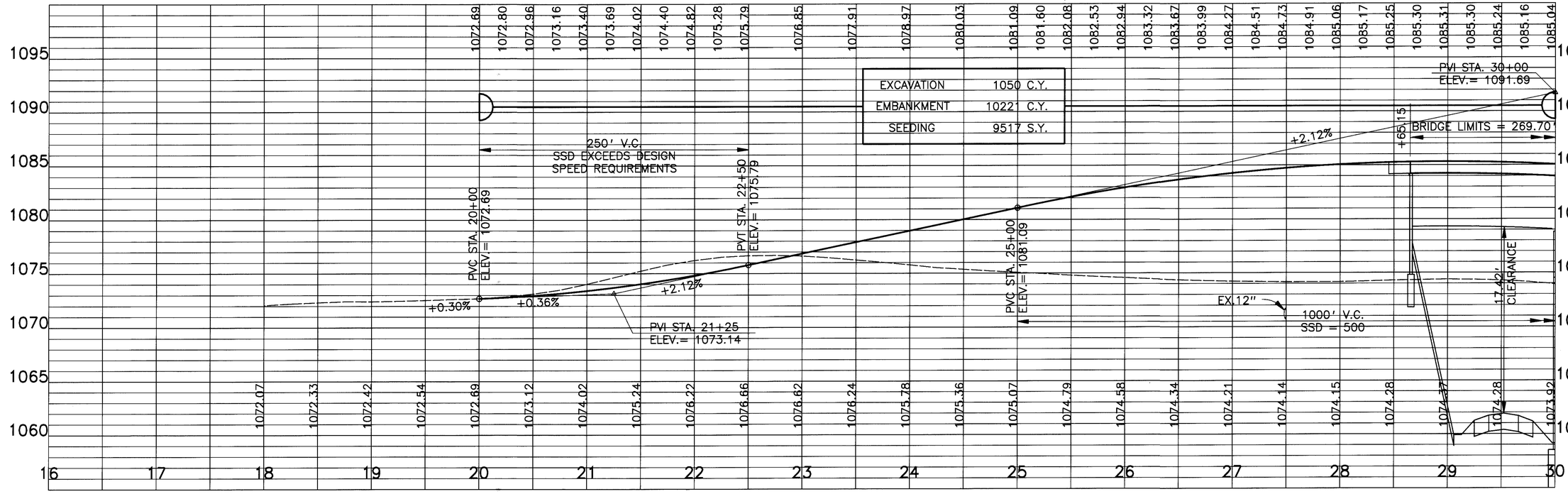
FOR TYPICAL SECTIONS, SEE SHEET 8
FOR CROSS–SECTIONS, SEE SHEETS 164–168
FOR QUANTITIES, SEE SHEETS 37,38,39,42

FOR DRIVE DETAILS, SEE SHEET 11
FOR APPROACH SLAB DETAILS, SEE SHEET 260

FOR BRIDGE NO. FAY–35–0308, SEE SHEETS 404–416

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[I A] IMPACT ATTENUATOR, TYPE 1–98 (BIDIRECTIONAL)
[A] ANCHOR ASSEMBLY, TYPE A
[T] ANCHOR ASSEMBLY, TYPE T

EXCAVATION	1050 C.Y.
EMBANKMENT	1022 C.Y.
SEEDING	9517 S.Y.



PLAN & PROFILE C.R. 16
STA. 16 + 00 TO STA. 30 + 00

FAY–35–2.57

SEE SHEET 162
MATCH LINE STA. 30 + 00

STA. 33 + 64.04 C.R.16
Δ = 0°-03'-00.9"LT.=
STA. 0 + 00.00 T.R.94

END APPROACH SLAB
STA. 31+54.85

PAVEMENT TAPER LT.
55:1

SHOULDER TAPER LT.& RT.
25:1

PAVEMENT TAPER RT.
55:1

END WORK
STA. 40+00

3 STY FR.GARAGE
WALL
WELL
WIND MILL
FIELD DR.
10"CMP
EX.R/W
Constr.Limits
LA R/W
R/W
WIRE FENCE
310'x8'
310'x11'
N.01°-09'-46.4"E.
N.01°-06'-45.5"E.
℄ CONSTR.
T.R.94
50'R.
S.87°-37'-25.7"E.

FOR TYPICAL SECTIONS, SEE SHEET 8
FOR CROSS-SECTIONS, SEE SHEETS 168-173
FOR QUANTITIES, SEE SHEETS 37,38,39,41,42

FOR PAVEMENT DETAILS, SEE SHEET 274
FOR DRIVE DETAILS, SEE SHEET 11
FOR APPROACH SLAB DETAILS, SEE SHEET 260

FOR T.R.94 - SEE SHEET 174

FOR BRIDGE NO. FAY-35-0308, SEE SHEETS 404-416

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[1A] IMPACT ATTENUATOR, TYPE 1-98 (BIDIRECTIONAL)
[A] ANCHOR ASSEMBLY, TYPE A
[T] ANCHOR ASSEMBLY, TYPE T

BM-TR 201
IRON PIN WITH CAP
STA. 30+01.34, 14.24'LT.
ELEV.= 1073.17

BM-TR 300
IRON PIN WITH CAP
STA. 42+71.86, 14.21'LT.
ELEV.= 1060.89

EXCAVATION	750 C.Y.
EMBANKMENT	10,591 C.Y.
SEEDING	9629 S.Y.

PVI STA. 30+00
ELEV.= 1091.69

BRIDGE LIMITS = 269.70'

17.06' CLEARANCE

-3.20%

1000' V.C.
SSD = 500

UGT

PVT STA. 35+00
ELEV.= 1075.69

PVC STA. 36+00
ELEV.= 1072.49

300' V.C.
SSD EXCEEDS DESIGN
SPEED REQUIREMENTS

PVI STA. 37+50
ELEV.= 1067.69

-0.92%

-0.86%

PVT STA. 39+00
ELEV.= 1066.31

34 L.F. 12" CONDUIT, TYPE D, ● 0.40%

HORIZONTAL SCALE IN FEET
0 25 50 100

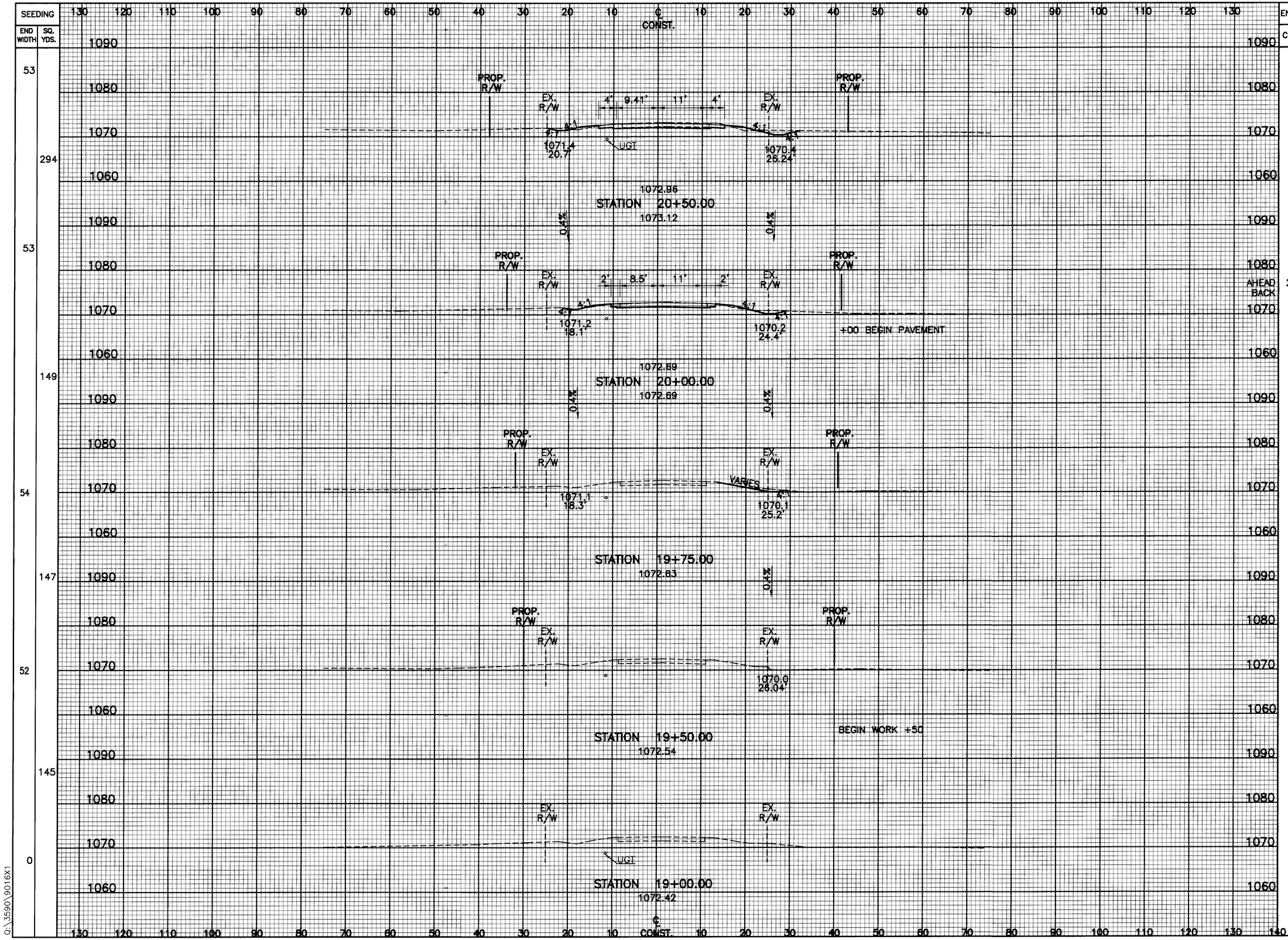
CALCULATED
CHECKED

PLAN & PROFILE C.R. 16
STA. 30 + 00 TO STA. 40 + 00

FAY-35-2.57

163
522

SEEDING END WIDTH	SEEDING SQ. YDS.
53	294
53	149
54	147
52	145
0	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
37	5		
		58	8
25	3		
4	3		
		4	2
4	0		
		2	0
0	0		

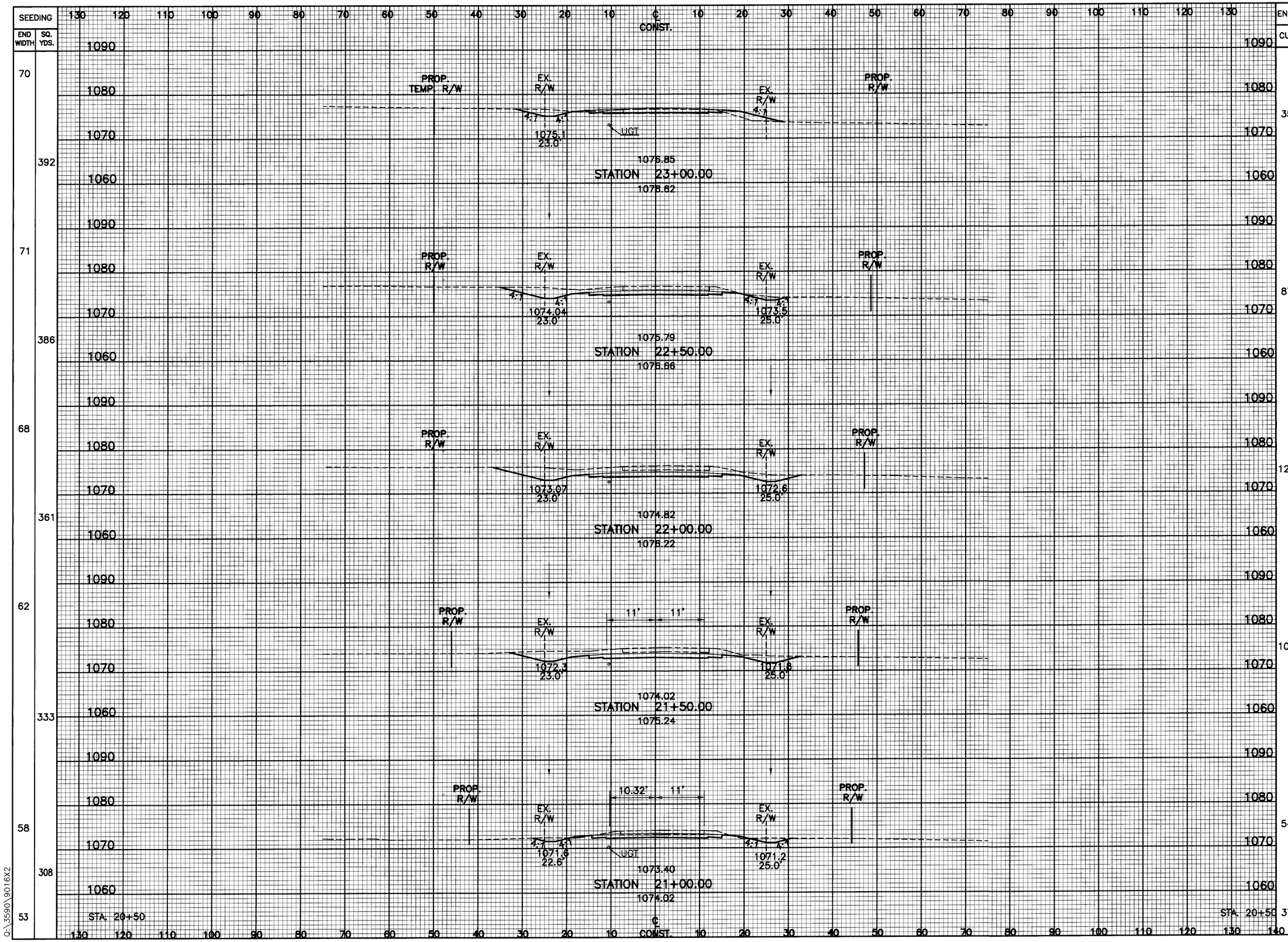
CALCULATED J.L.G.
CHECKED W.G.Y.

C.R. 16 X-SECTIONS STATION 19+00 TO STATION 20+50

FAY-35-2.57

Q:\3590\9016X1

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
70	392	35	14	113	14	
71	386	87	1	195	1	
68	361	123	0	211	0	
62	333	104	0	147	3	
58	308	54	3	85	8	
53		37	5			

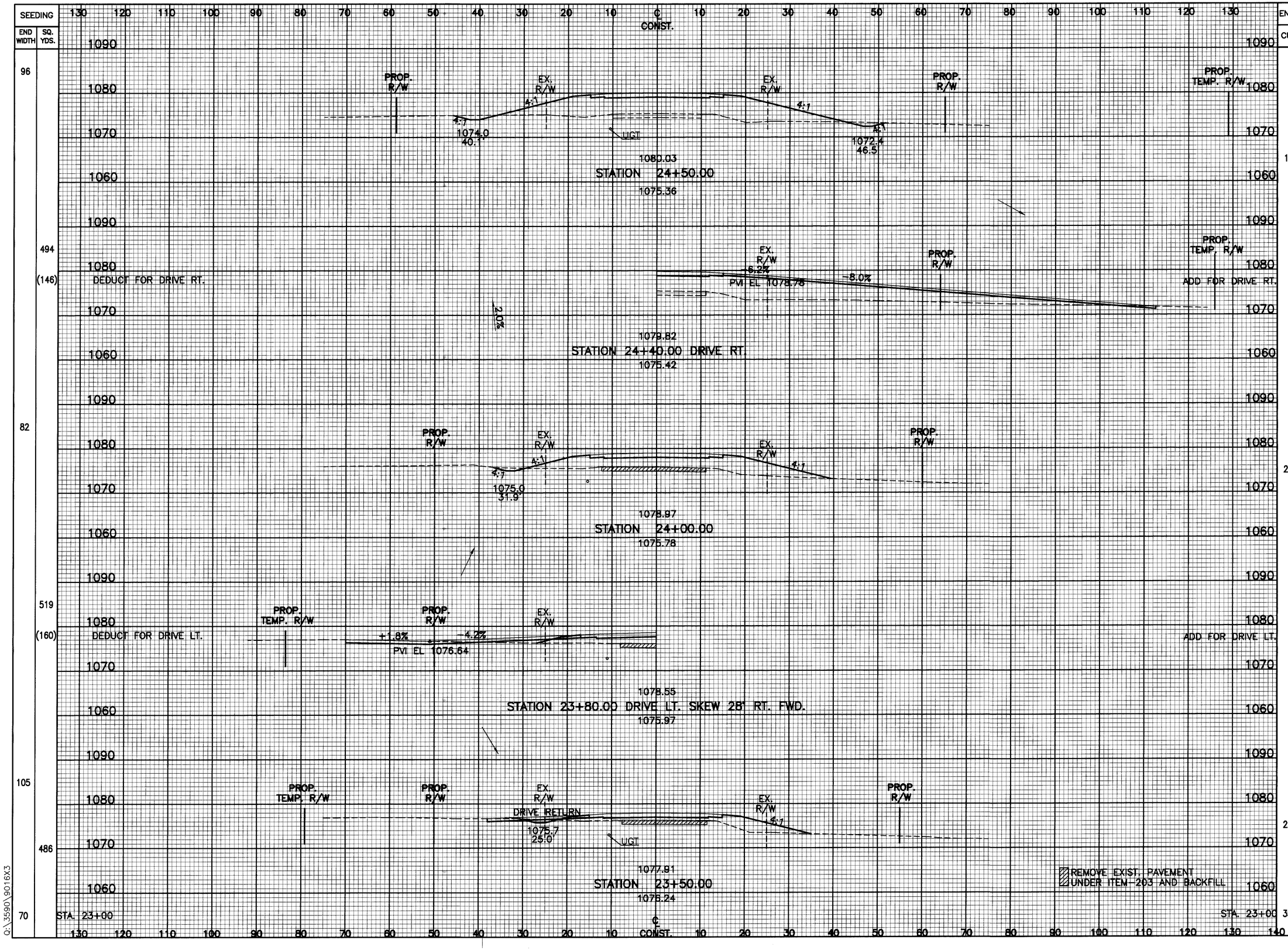


C.R. 16 X-SECTIONS STATION 21+00 TO STATION 23+00

FAY-35-2.57

Q:\3590\9016X2

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
96		11	259			
	494			34	381	
	(146)			(2)	(289)	
82		25	174			
	519			44	238	
	(160)			(13)	(4)	
105		22	83			
	486			53	90	
70		35	14			

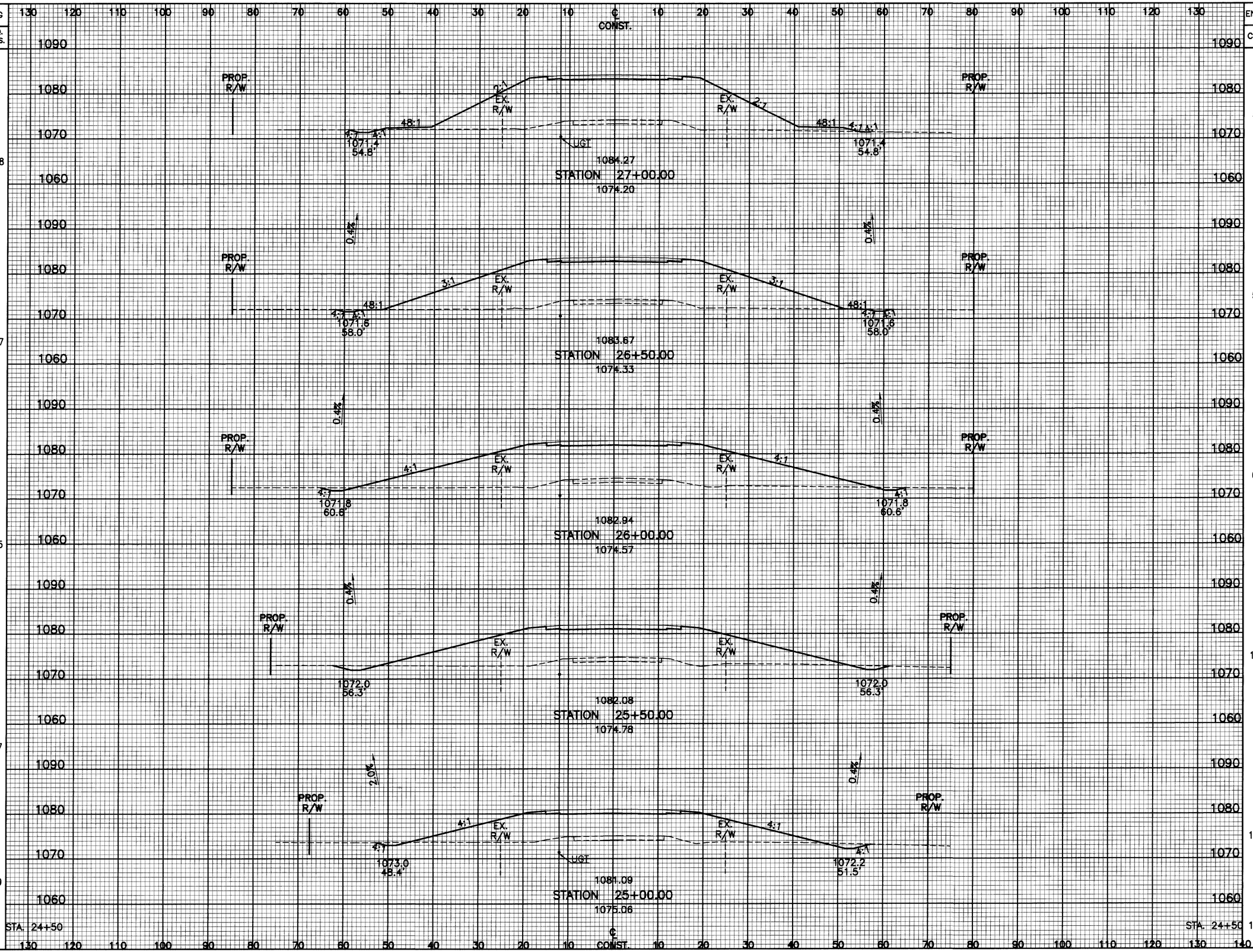


C.R. 16 X-SECTIONS STATION 23+50 TO STATION 24+50

FAY-35-2.57

Q:\3590\9016X3

SEEDING END WIDTH	SEEDING SQ. YDS.
141	778
139	767
137	725
124	647
109	569
96	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
4	645		
		9	1229
5	682		
		11	1256
6	674		
		16	1121
11	536		
		20	858
10	390		
		20	601
11	259		

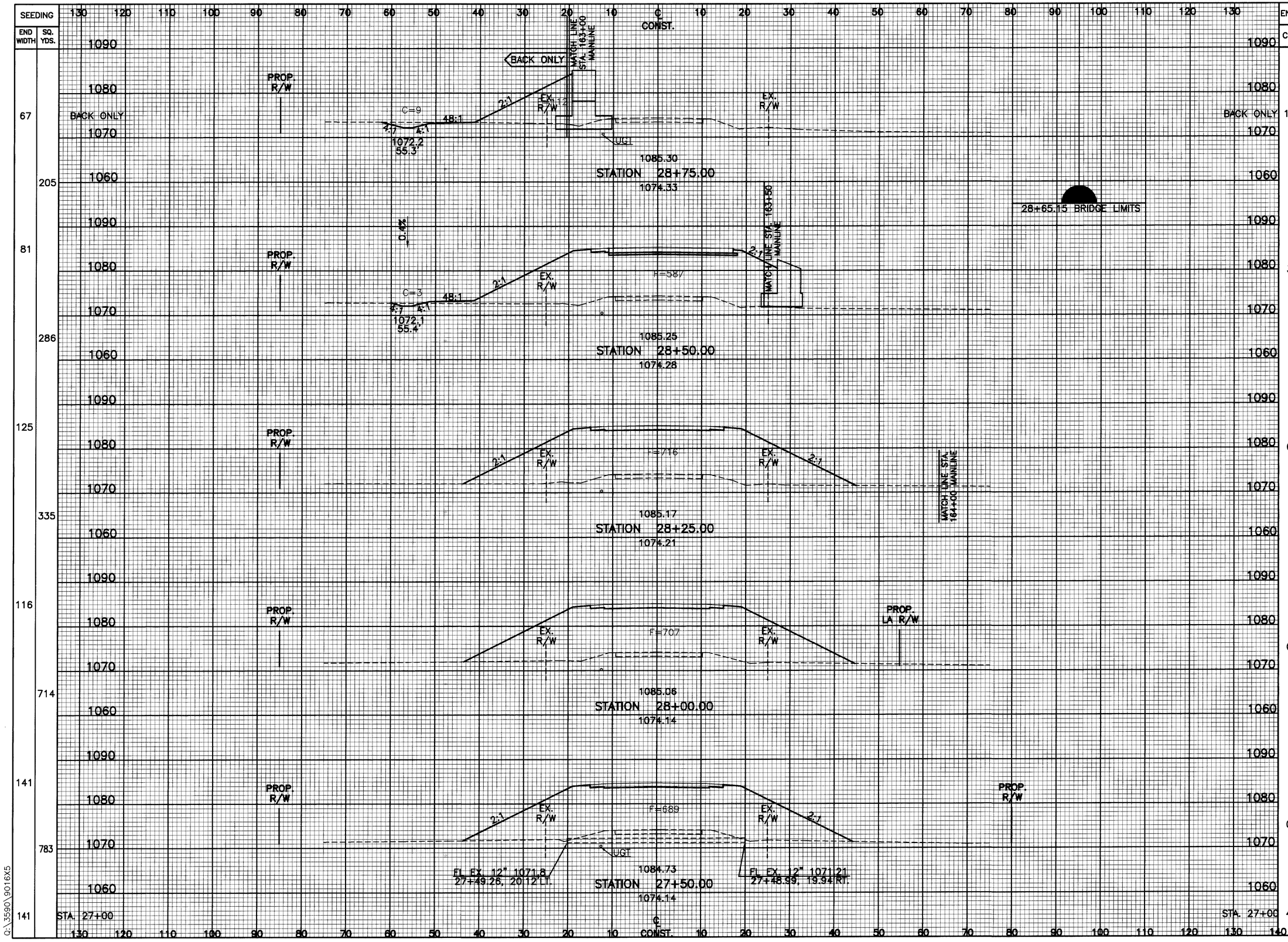
CALCULATED J.L.G.
CHECKED W.G.Y.

C.R. 16 X-SECTIONS STATION 25+00 TO STATION 27+00

FAY-35-2.57

Q:\3590\9016X4

SEEDING END WIDTH	SEEDING SQ. YDS.
67	205
81	286
125	335
116	714
141	783
141	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
10	113		
		7	325
3	588		
		2	604
0	717		
		0	660
0	707		
		0	1293
0	689		
		4	1236
4	645		

CALCULATED J.L.G.
CHECKED W.G.Y.

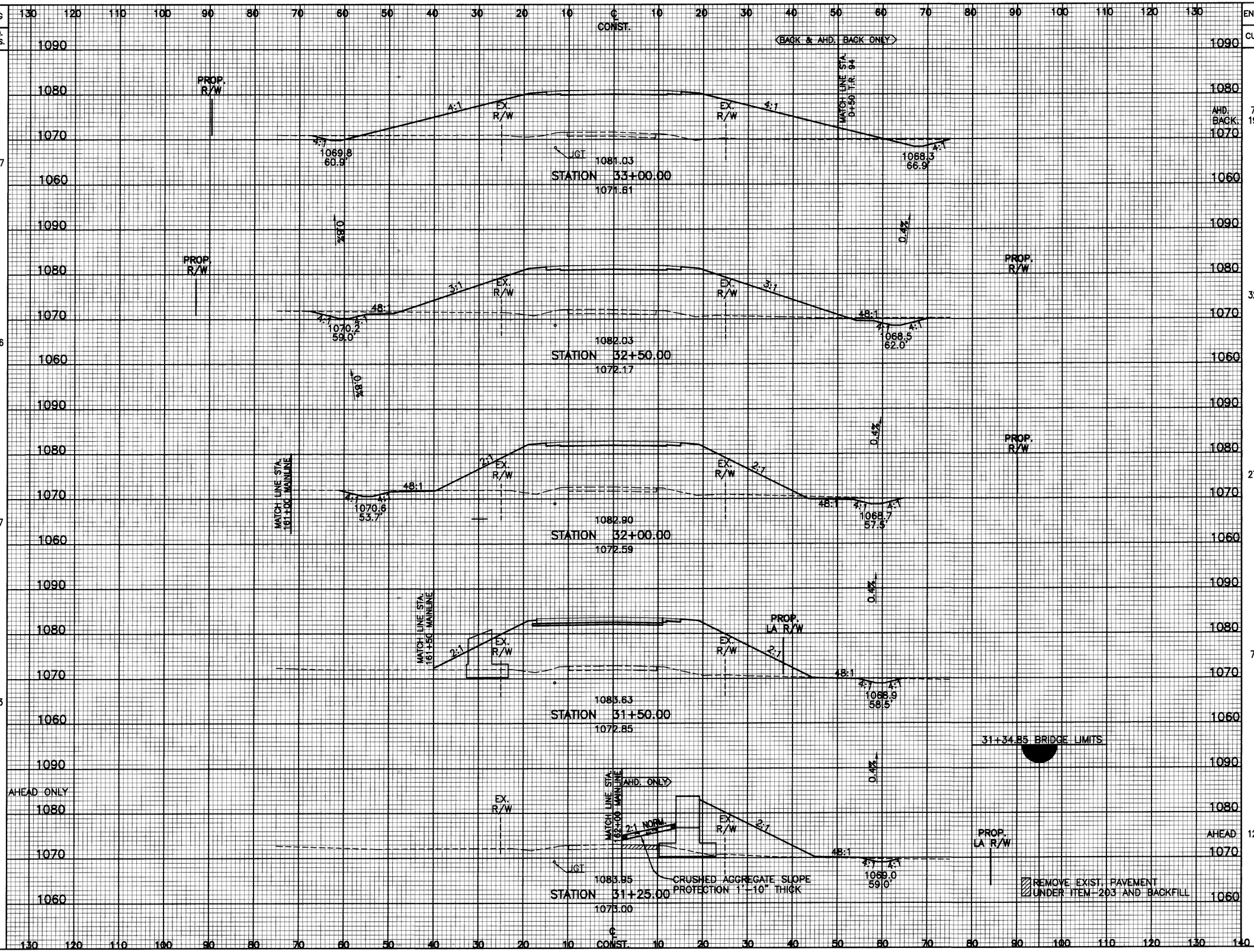
C.R. 16 X-SECTIONS STATION 27+50 TO STATION 28+75

FAY-35-2.57

168
522

Q:\3590\9016X5

SEEDING END WIDTH	SEEDING SQ. YDS.
112	747
157	816
137	667
103	263
86	AHEAD ONLY



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
7	720		
19	732		
		48	1301
32	673		
		55	1190
27	612		
		32	1163
7	643		
		9	412
AHEAD 12	245		

CALCULATED J.L.G.
CHECKED W.G.Y.

C.R. 16 X-SECTIONS STATION 31+25 TO STATION 33+00

FAY-35-2.57

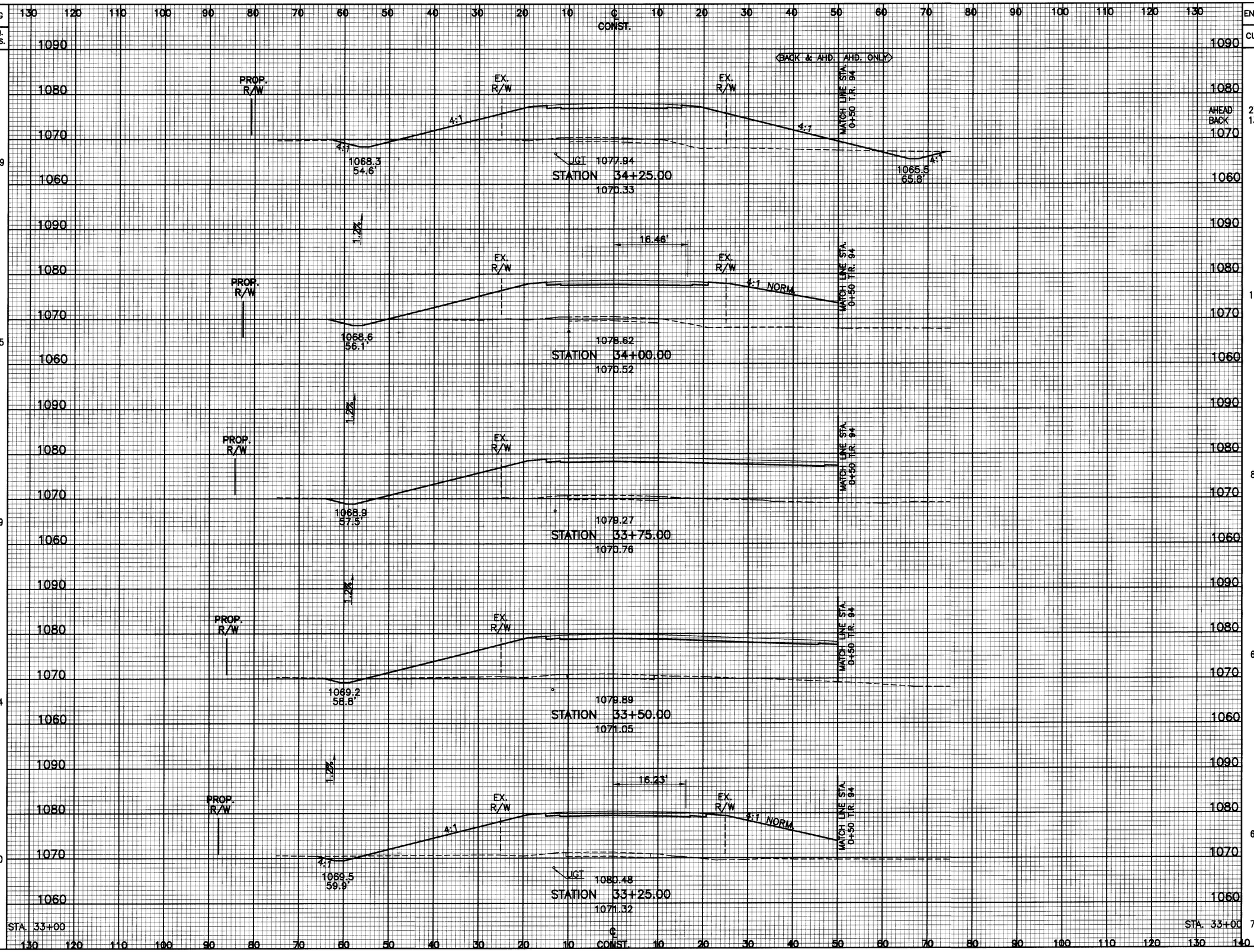
Q:\3590\9016X6

SEEDING END WIDTH	SEEDING SQ. YDS.
103	279
98	235
71	199
72	244
104	300
112	

STA. 33+00

130 120 110 100 90 80 70 60 50 40 30 20 10 **℄ CONST.** 10 20 30 40 50 60 70 80 90 100 110 120 130 140

1090 / 1080 / 1070 / 1060



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
AHEAD 27 / BACK 13	561 / 553		
		12	555
11	645		
		9	619
8	690		
		7	651
6	715		
		6	666
6	723		
		7	669
7	720		

CALCULATED J.L.G.
CHECKED W.G.Y.

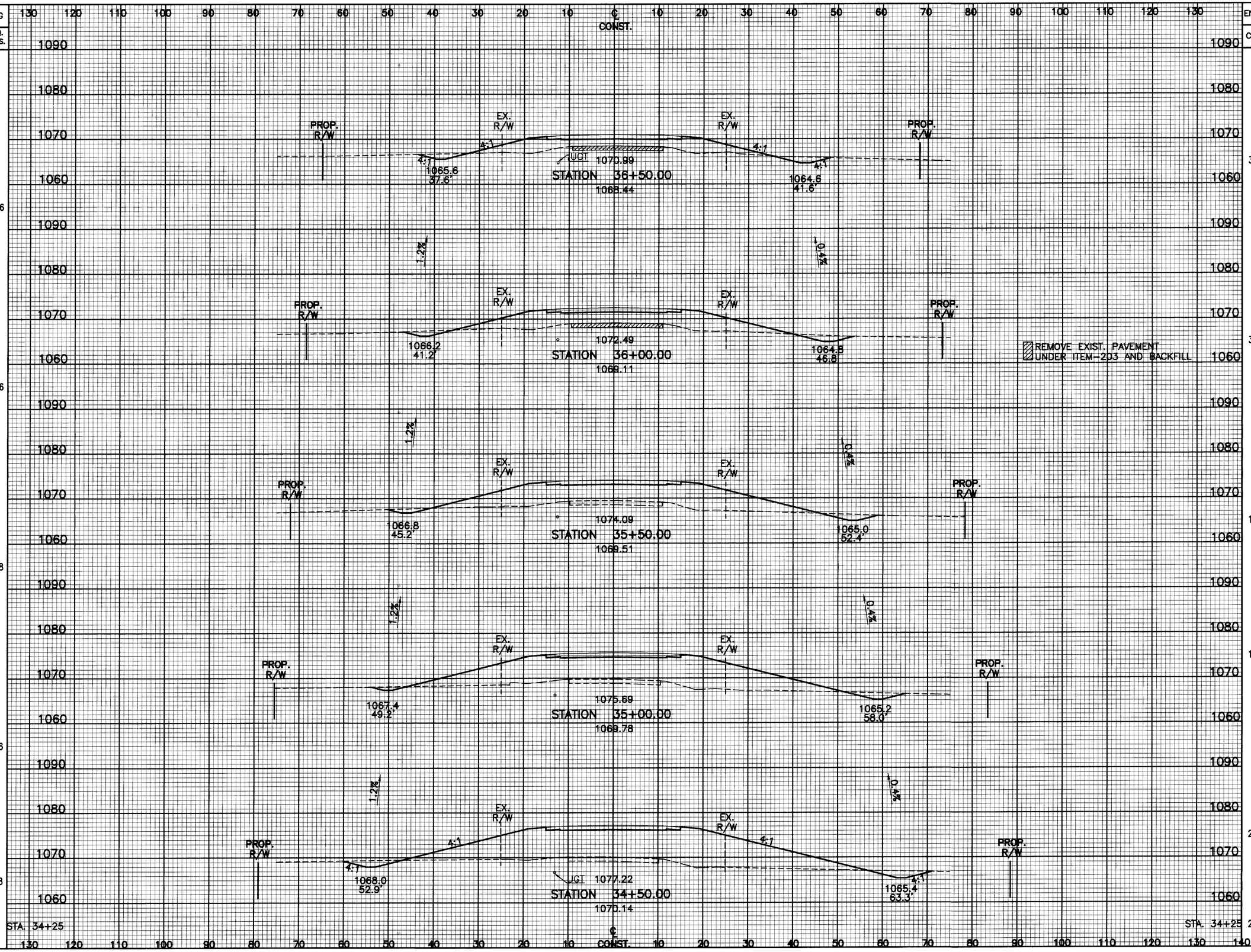
C.R. 16 X-SECTIONS STATION 33+25 TO STATION 34+25

FAY-35-2.57

170 / 522

Q:\3590\9016X7

SEEDING END WIDTH	SEEDING SQ. YDS.
105	606
113	656
123	708
132	756
140	338
103	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
36	144		
		68	326
37	208		
		50	462
16	290		
		29	649
15	410		
		37	850
24	508		
		24	495
27	561		

CALCULATED J.L.G.
CHECKED W.G.Y.

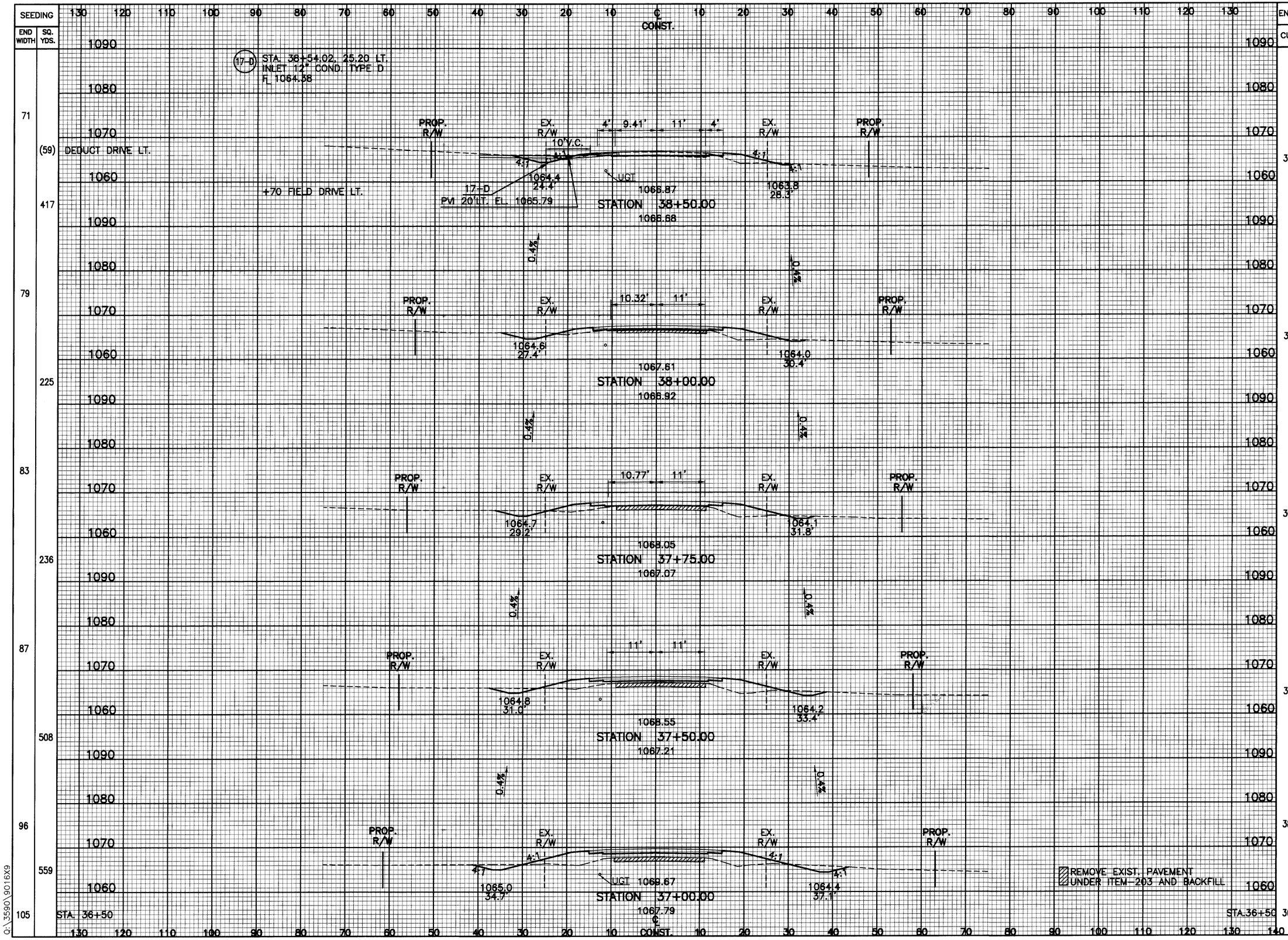
C.R. 16 X-SECTIONS STATION 34+50 TO STATION 36+50

FAY-35-2.57

171
522

Q:\3590\9016X8

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
71	(59)	34	21	61	61	
79	417	31	44			
83	225	30	61	29	49	
87	236	31	79	29	65	
96	508	38	106	64	172	
105	559	36	144	69	232	



C.R. 16 X-SECTIONS STATION 37+00 TO STATION 38+50

FAY-35-2.57

172
522

Q:\3590\9016X9

SEEDING	
END WIDTH	SQ. YDS.
0	
33	
	192
36	
62	
	356
66	
	381
71	

END AREA		VOLUME		CALCULATED	J.L.G.
CUT	FILL	CUT	FILL	CHECKED	W.G.Y.
0	0				
		9	0		
9	0				
		29	2		
22	2				
38	2				
		67	2		
34	21				

C.R. 16 X-SECTIONS STATION 39+00 TO STATION 40+00

FAY-35-2.57

173
522

Q:\3590\9016X10

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

1090
1080
1070
1060

AHEAD
BACK

PROP. R/W
EX. R/W
EX. R/W
PROP. R/W

1063.8
21.2
UGT
STATION 40+00.00
1065.61
+00 END WORK

0.4%

PROP. R/W
EX. R/W
EX. R/W
PROP. R/W

4:1
4:1
1064.0
21.0
1063.4
18.8
STATION 39+50.00
1065.88

0.4%
0.4%

2' 8.5' 11.75' 2'

PROP. R/W
EX. R/W
EX. R/W
PROP. R/W

4:1
4:1
4:1
4:1
UGT 1066.31
1064.2
21.9
1063.5
21.5
STATION 39+00.00
1066.31
+00 END NEW PAVEMENT

AHEAD
BACK

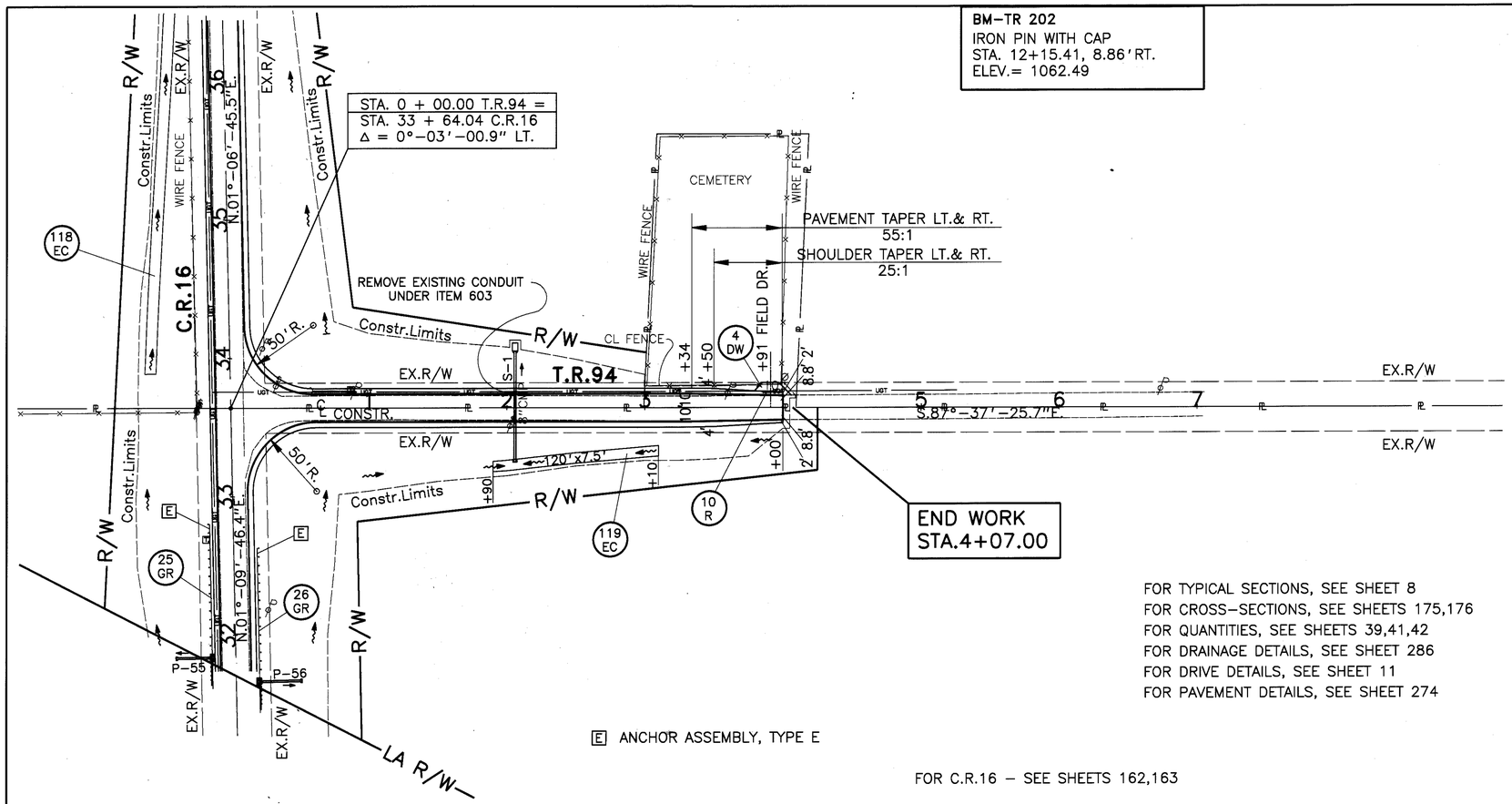
STA. 38+50

17-0 STA. 38+87.98, 23.50 LT.
OUTLET 12" CONDUIT TYPE D
FL 064.25

STA. 38+50

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130 140

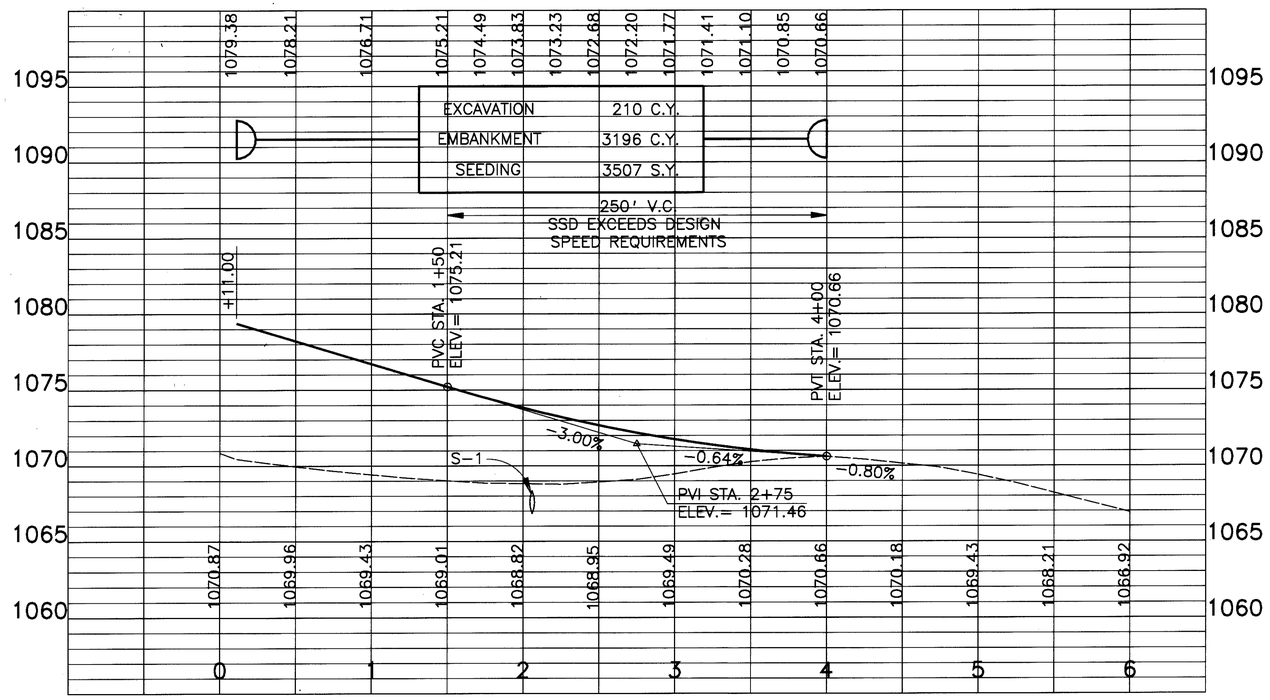
BM–TR 202
IRON PIN WITH CAP
STA. 12+15.41, 8.86'RT.
ELEV.= 1062.49



[E] ANCHOR ASSEMBLY, TYPE E

FOR TYPICAL SECTIONS, SEE SHEET 8
FOR CROSS–SECTIONS, SEE SHEETS 175,176
FOR QUANTITIES, SEE SHEETS 39,41,42
FOR DRAINAGE DETAILS, SEE SHEET 286
FOR DRIVE DETAILS, SEE SHEET 11
FOR PAVEMENT DETAILS, SEE SHEET 274

FOR C.R.16 – SEE SHEETS 162,163



PLAN & PROFILE T.R. 94
STA. 0 + 00 TO STA. 5 + 00

FAY–35–2.57

174
522

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
84	509	18	186	22	424	
99	587	5	271	12	579	
112	664	7	354	18	743	
127	620	12	448	12	1093	
96		0	732			

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130 140

1090 1080 1070 1060

PROP. R/W · EX. R/W · EX. R/W · PROP. R/W

4:1 · 4:1 · 4:1

UGT

1072.68

STATION 2+50.00

1068.85

1067.3
35.2'

REMOVE EXIST. PAVEMENT UNDER ITEM 203 AND BACKFILL

0.8%

HEADWALL HW-2.1M OR 2.2M
STA 2+06, 41' LT.
℄ OUTLET =1068.58

HEADWALL HW-2.1M OR 2.2M
STA. 2+06, 39' RT.
℄ INLET = 1067.09

S-1

R.C.P. TYPE C

FL EX.10" 1067.25
2+06.30, 11.99' LT.
REMOVE UNDER ITEM 603

FL EX.10" 1067.27
2+16.38, 11.67' RT.
REMOVE UNDER ITEM 603

1067.1
40.6'

1073.83

STATION 2+00.00

1068.82

1.0%

1075.21

1067.6
44.1'

STATION 1+50.00

1069.01

1.0%

1075.71

1068.1
48.1'

STATION 1+00.00

1068.43

11' · 11.5'

4:1 NORM. · 4:1 NORM.

MATCH LINE STA. 34+25 C.R. 16

MATCH LINE STA. 33+00 C.R. 16

UGT

1075.21

STATION 0+50.00

1069.96

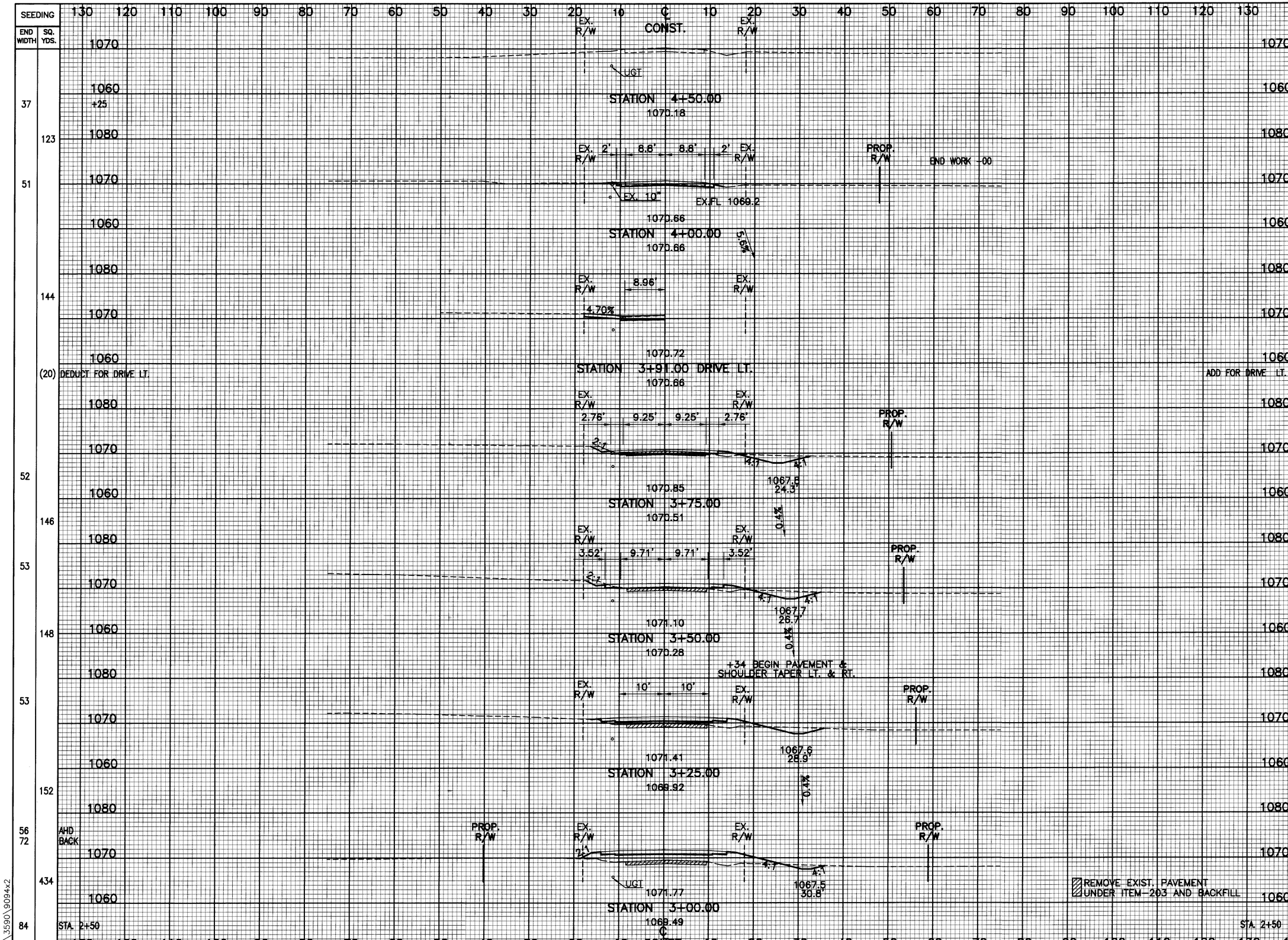
℄ CONST.

T.R. 94 X-SECTIONS STATION 0+50 TO STATION 2+50

FAY-35-2.57

175
522

Q:\3590\9094x1



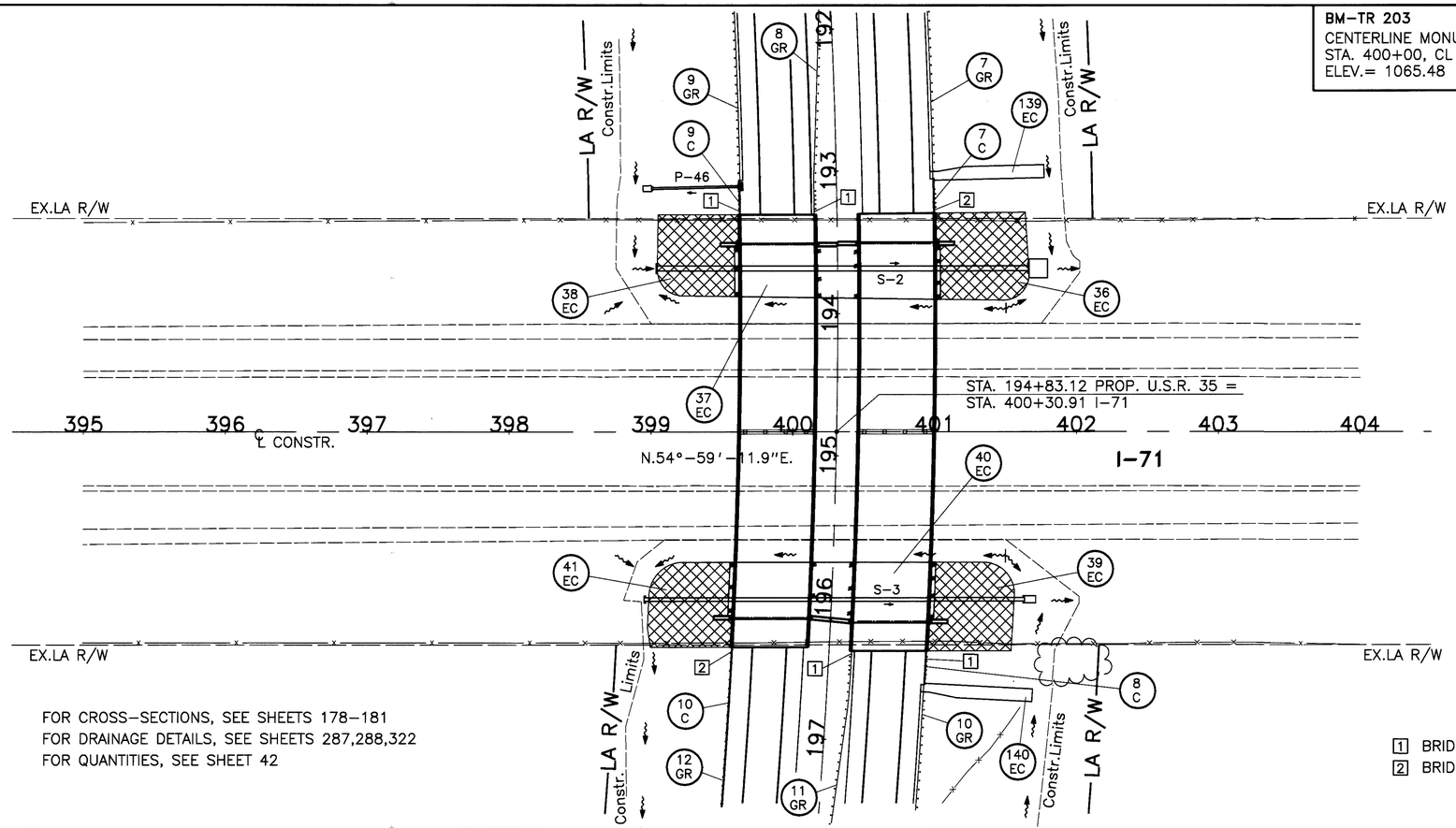
CALCULATED J.L.G.
CHECKED W.G.Y.

T.R. 94 X-SECTIONS STATION 3+00 TO STATION 4+50

FAY-35-2.57

Q:\3590\9094x2

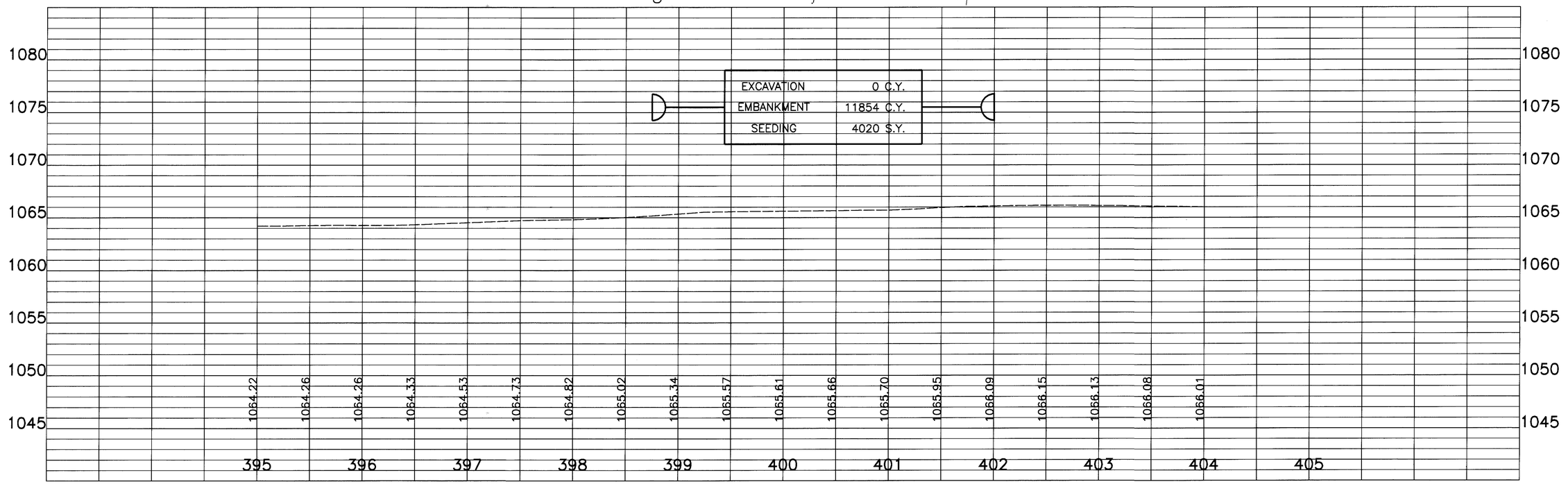
BM–TR 203
CENTERLINE MONUMENT
STA. 400+00, CL
ELEV.= 1065.48



FOR CROSS–SECTIONS, SEE SHEETS 178–181
FOR DRAINAGE DETAILS, SEE SHEETS 287,288,322
FOR QUANTITIES, SEE SHEET 42

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[2] BRIDGE TERMINAL ASSEMBLY, TYPE 2

EXCAVATION	0 C.Y.
EMBANKMENT	11854 C.Y.
SEEDING	4020 S.Y.



HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE I.R. 71
STA. 395 + 00 TO STA. 404 + 00

FAY–35–2.57

SEEDING	
END WIDTH	SQ. YDS.
114	
	198
28	

+75 AHD

END AREA		VOLUME		CALCULATED
CUT	FILL	CUT	FILL	J.L.G.
0	44			CHECKED
		0	21	W.G.Y.
0	0			

260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080
1060
1040

EX. LA R/W

MATCH LINE STA. 193+50 MAINLINE

S-2

HEADWALL HW-2.1M OR 2.2M
STA 399+03.26, 115 LT
℄ INLET = 1057.25

S-3

HEADWALL HW-2.1M OR 2.2M
STA 398+95.54, 118' RT
℄ INLET = 1056.71

MATCH LINE STA. 196+25 MAINLINE

STATION 399+00.00
1065.34

BEGIN WORK +75

STATION 398+75.00
1065.16

STATION 398+50.00
1065.02

STATION 398+00.00
1064.82

STATION 397+50.00
1064.73

℄ CONST.

I-71 X-SECTIONS STA. 397+50 TO 399+00

FAY-35-2.71

Q:\3590\90171X1

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
130	684	STATION 401+00.00 1065.70	0	1604	0	2803
116	642	STATION 400+50.00 1065.66	0	1423	0	2584
115	662	STATION 400+00.00 1065.61	0	1367	0	2450
123	337	STATION 399+50.00 1065.57	0	1279	0	972
119	324	STATION 399+25.00 1065.53	0	820	0	400
114		STA.399+00	0	44		

CALCULATED J.L.G.
CHECKED W.G.Y.

I-71 X-SECTIONS STA. 399+25 TO STA. 401+00

FAY-35-2.57

179
522

Q:\3590\90I71X2

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
33	155	0	0	0	7	
78	288	0	13	0	219	
129	364	0	615	0	934	
133	366	0	1401	0	1392	
130		0	1604			

260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 1060 1040

EX. LA R/W

STATION 402+50.00

STATION 402+00.00
1066.09

END WORK +00

MATCH LINE STA. 193+50 MAINLINE

MATCH LINE STA. 196+25 MAINLINE

2:1

1056.9
114.0'

1056.7
117.0'

STATION 401+75.00
1068.05

S-2
HEADWALL HW=2.1M OR 2.2M
STA 401+66.26, 1.15' LT
E OUTLET = 1056.93

S-3
HEADWALL HW=2.1M OR 2.2M
STA 401+62.54, 118' RT
E OUTLET = 1056.55

1063.2
86.75'

1063.2
86.75'

STATION 401+50.00
1065.95

0.4%

2:1 NORM.

1063.1
86.75'

1063.1
86.75'

STATION 401+25.00
1065.81

STA. 401+00

STA. 401+00

I-71 X-SECTIONS STA. 401+25 TO 402+50

FAY-35-2.71

180
522

Q:\3590\90171X3

SEEDING	
END WIDTH	SQ. YDS.

END AREA		VOLUME		CALCULATED
CUT	FILL	CUT	FILL	J.L.G.
				CHECKED
				W.G.Y.

260 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

1080 1060 1040

EX. LA R/W

STATION 403+25.00

25:1 25:1

STATION 403+00.00

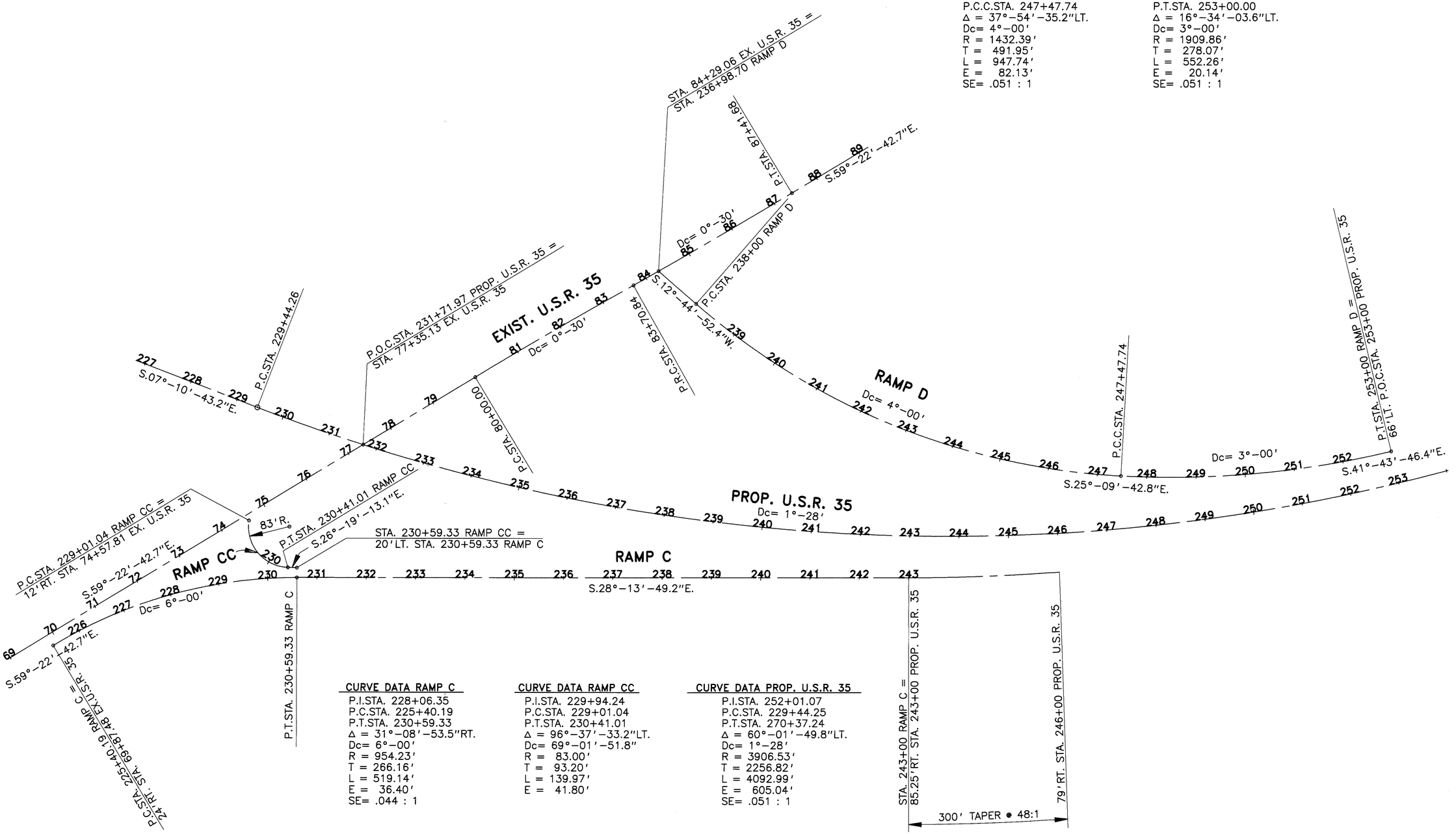
STATION 402+75.00

280 240 220 200 180 160 140 120 100 80 60 40 20 ℄ CONST. 20 40 60 80 100 120 140 160 180 200 220 240

I-71 X-SECTIONS STA. 402+75 TO 403+25

FAY-35-2.71

Q:\3590\90171X4



CURVE DATA RAMP D
P.I.STA. 242+91.95
P.C.STA. 238+00.00
P.C.C.STA. 247+47.74
Δ = 37°–54'–35.2"LT.
Dc= 4°–00'
R = 1432.39'
T = 491.95'
L = 947.74'
E = 82.13'
SE= .051 : 1

CURVE DATA RAMP D
P.I.STA. 250+25.81
P.C.C.STA. 247+47.74
P.T.STA. 253+00.00
Δ = 16°–34'–03.6"LT.
Dc= 3°–00'
R = 1909.86'
T = 278.07'
L = 552.26'
E = 20.14'
SE= .051 : 1

CURVE DATA RAMP C
P.I.STA. 228+06.35
P.C.STA. 225+40.19
P.T.STA. 230+59.33
Δ = 31°–08'–53.5"RT.
Dc= 6°–00'
R = 954.23'
T = 266.16'
L = 519.14'
E = 36.40'
SE= .044 : 1

CURVE DATA RAMP CC
P.I.STA. 229+94.24
P.C.STA. 229+01.04
P.T.STA. 230+41.01
Δ = 96°–37'–33.2"LT.
Dc= 69°–01'–51.8"
R = 83.00'
T = 93.20'
L = 139.97'
E = 41.80'

CURVE DATA PROP. U.S.R. 35
P.I.STA. 252+01.07
P.C.STA. 229+44.25
P.T.STA. 270+37.24
Δ = 60°–01'–49.8"LT.
Dc= 1°–28'
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'
SE= .051 : 1

CALCULATED
CHECKED
HORIZONTAL SCALE IN FEET
0 50 100 200

U.S.R. 35 INTERCHANGE
GEOMETRIC LAYOUT

FAY–35–2.57

182
522

BM–TR 14
IRON PIN WITH CAP
STA. 65+74.70, 32.75′RT.
ELEV.= 1060.16

BM–TR 15
IRON PIN WITH CAP
STA. 71+84.05, 31.68′RT.
ELEV.= 1061.40

BM–TR 13
IRON PIN WITH CAP
STA. 59+81.32, 30.90′RT.
ELEV.= 1059.78

BEGIN WORK
STA. 62+25

STA. 65+75 EXIST. U.S.R. 35 =
STA. 10+00 SERVICE RD. NO.1 & NO.2

FOR TYPICAL SECTIONS, SEE SHEET 9
FOR CROSS–SECTIONS, SEE SHEETS 188–193
FOR QUANTITIES, SEE SHEETS 37,38,39,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 294,295
FOR DRIVE DETAILS, SEE SHEET 11
FOR PAVEMENT DETAILS, SEE SHEETS 267,268,275
FOR UNDERDRAIN DETAILS, SEE SHEET 353
FOR GEOMETRIC LAYOUT, SEE SHEET 182

[E] ANCHOR ASSEMBLY, TYPE E–98

FOR RAMP C, SEE SHEETS 207,208
FOR SERVICE ROADS, SEE SHEET 222

MATCH LINE STA. 73 + 00
SEE SHEET 184

EXCAVATION	1972 C.Y.
EMBANKMENT	38,542 C.Y.
SEEDING	16,774 S.Y.

450′ V.C.
SSD EXCEEDS DESIGN
SPEED REQUIREMENTS

PVC STA. 64+00
ELEV.= 1061.30

PVI STA. 66+25
ELEV.= 1062.47

PVI STA. 68+50
ELEV.= 1071.29

PVC STA. 69+00
ELEV.= 1073.25

1500′ V.C.
SSD = 504

+0.40%
+0.52%
+3.92%
+3.92%

EX.18″
S–7
S–7A
9–D

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE EX.U.S.R. 35
STA. 60 + 00 TO STA. 73 + 00

FAY–35–2.57

183
522

SEE SHEET 183
MATCH LINE STA. 73 + 00

MATCH LINE STA. 86 + 00
SEE SHEET 185

1 BRIDGE TERMINAL ASSEMBLY, TYPE 1
1A IMPACT ATTENUATOR, TYPE 1–98 (BIDIRECTIONAL)
E ANCHOR ASSEMBLY, TYPE E–98
T ANCHOR ASSEMBLY, TYPE T

CURVE DATA
P.I.STA. 81+85.44
P.C.STA. 80+00.00
P.R.C.STA. 83+70.84
Δ = 1°–51′–15.1″RT.
Dc= 0°–30′
R = 11,459.16′
T = 185.44′
L = 370.84′
E = 6.00′

CURVE DATA
P.I.STA. 85+56.28
P.R.C.STA. 83+70.84
P.T.STA. 87+41.68
Δ = 1°–51′–15.1″LT.
Dc= 0°–30′
R = 11,459.16′
T = 185.44′
L = 370.84′
E = 6.00′

SERVICE ROAD NO.3

TEMP.RD.

LA R/W

Constr.Limits

74+16,153′LT.
INV. 1052.0

74+28,141′LT.
INV. 1053.0

78+12,134′LT.
INV. 1050.0

78+24,122′LT.
INV. 1051.0

396′–24″SLEEVE

396′–12″SLEEVE

35′x7.5′

BEGIN APPROACH SLAB
STA. 75+84.13

END APPROACH SLAB
STA. 78+86.13

STA. 84+29.06 EX. U.S.R. 35 =
STA. D 236+98.70

EX.R/W

P.C.STA. CC 229+01.04

P.T.STA. CC 230+41.01

RAMP CC

RAMP C

RAMP D

P.C.STA. D 238+00

120′x7.5′

P.T.STA. C 230+59.33

STA. 77+35.12 EX. U.S.R. 35 =
STA. 231+71.97 PROP. U.S.R. 35

PVI STA. 76+50
ELEV.= 1102.65

BM92–TR 2
IRON PIN WITH CAP
STA. 76+57.87, 31.40′RT.
ELEV.= 1061.23

FOR TYPICAL SECTIONS, SEE SHEET 9
FOR CROSS–SECTIONS, SEE SHEETS 193–201
FOR QUANTITIES, SEE SHEETS 37,38,39,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 296,309,318,322
FOR DRIVE DETAILS, SEE SHEET 11
FOR PAVEMENT DETAILS, SEE SHEETS 268,272
FOR UNDERDRAIN DETAILS, SEE SHEET 353
FOR GEOMETRIC LAYOUT, SEE SHEET 182
FOR APPROACH SLAB DETAILS, SEE SHEET 262
FOR BRIDGE NO. FAY–35–0435, SEE SHEETS 417–436

FOR RAMP C, SEE SHEETS 207,208
FOR RAMP D, SEE SHEET 216
FOR SERVICE RD. NO.3, SEE SHEETS 229,230

BRIDGE LIMITS = 252.00′

+3.92%

−3.92%

+09.13

+61.13

17.04′ CLEARANCE

18.20′ CLEARANCE

1500′ V.C.
SSD = 504

500′ V.C.
SSD = 510

PVC STA. 85+00
ELEV.= 1069.33

PVT STA. 84+00
ELEV.= 1073.25

EX.12″

S–8

INV 24″ – 1052.0
INV 12″ – 1053.0

396′ – SLEEVES @ 0.5%

INV 24″ – 1050.0
INV 12″ – 1051.0

EXCAVATION	297 C.Y.
EMBANKMENT	93,224 C.Y.
SEEDING	16,209 S.Y.

1095 1090 1085 1080 1075 1070 1065 1060 1055

73 74 75 76 77 78 79 80 81 82 83 84 85 86

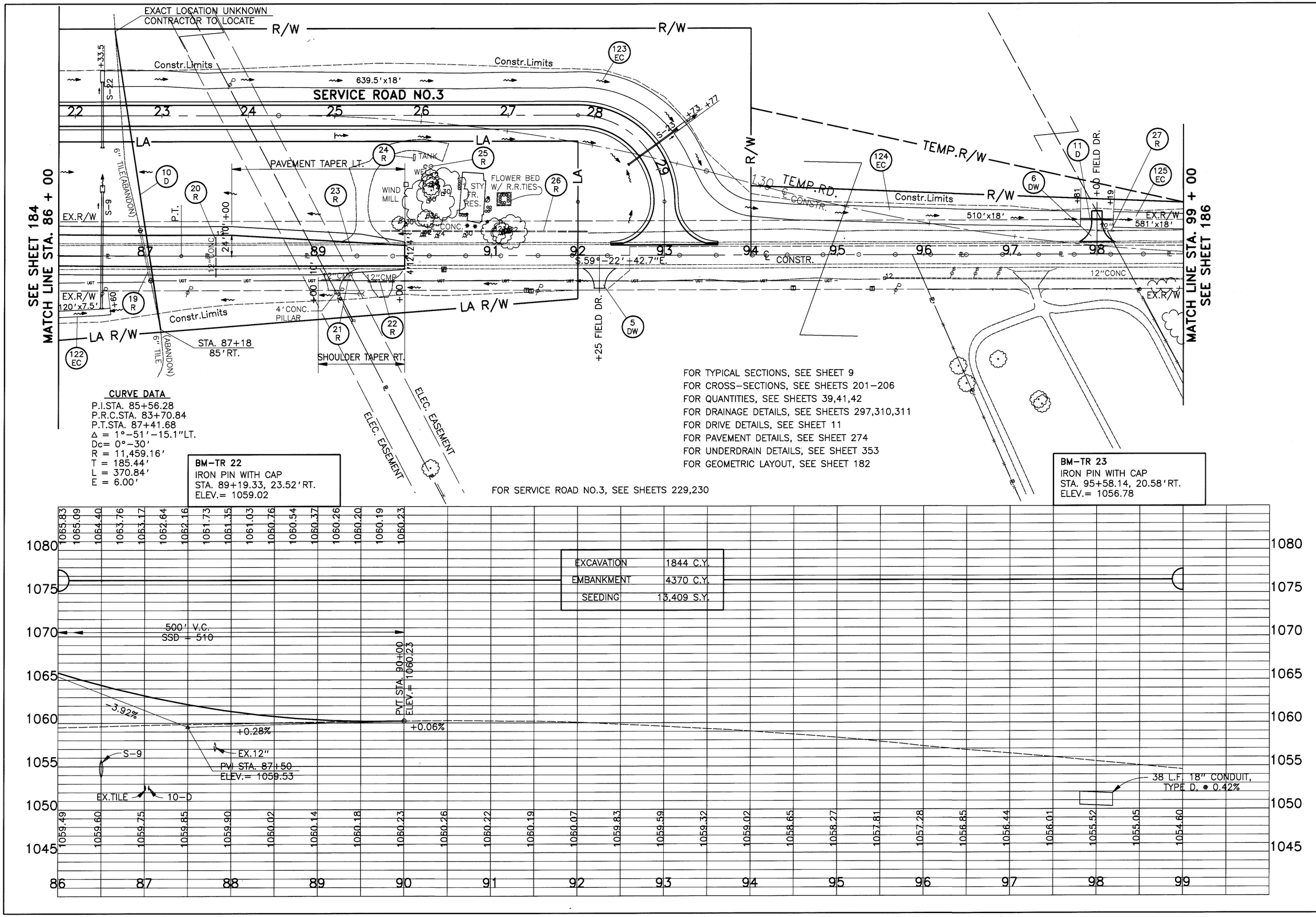
REVISED 1/00 BY FYC
CALCULATED
CHECKED

HORIZONTAL SCALE IN FEET
0 25 50 100

PLAN & PROFILE EX.U.S.R. 35
STA. 73 + 00 TO STA. 86 + 00

FAY–35–2.57

184
522



CURVE DATA

P.I.STA. 85+56.28
P.R.C.STA. 83+70.84
P.T.STA. 87+41.68
Δ = 1°-51'-15.1"LT.
Dc= 0°-30'
R = 11,459.16'
T = 185.44'
L = 370.84'
E = 6.00'

BM-TR 22
IRON PIN WITH CAP
STA. 89+19.33, 23.52'RT.
ELEV.= 1059.02

FOR TYPICAL SECTIONS, SEE SHEET 9
FOR CROSS-SECTIONS, SEE SHEETS 201-206
FOR QUANTITIES, SEE SHEETS 39,41,42
FOR DRAINAGE DETAILS, SEE SHEETS 297,310,311
FOR DRIVE DETAILS, SEE SHEET 11
FOR PAVEMENT DETAILS, SEE SHEET 274
FOR UNDERDRAIN DETAILS, SEE SHEET 353
FOR GEOMETRIC LAYOUT, SEE SHEET 182

FOR SERVICE ROAD NO.3, SEE SHEETS 229,230

BM-TR 23
IRON PIN WITH CAP
STA. 95+58.14, 20.58'RT.
ELEV.= 1056.78

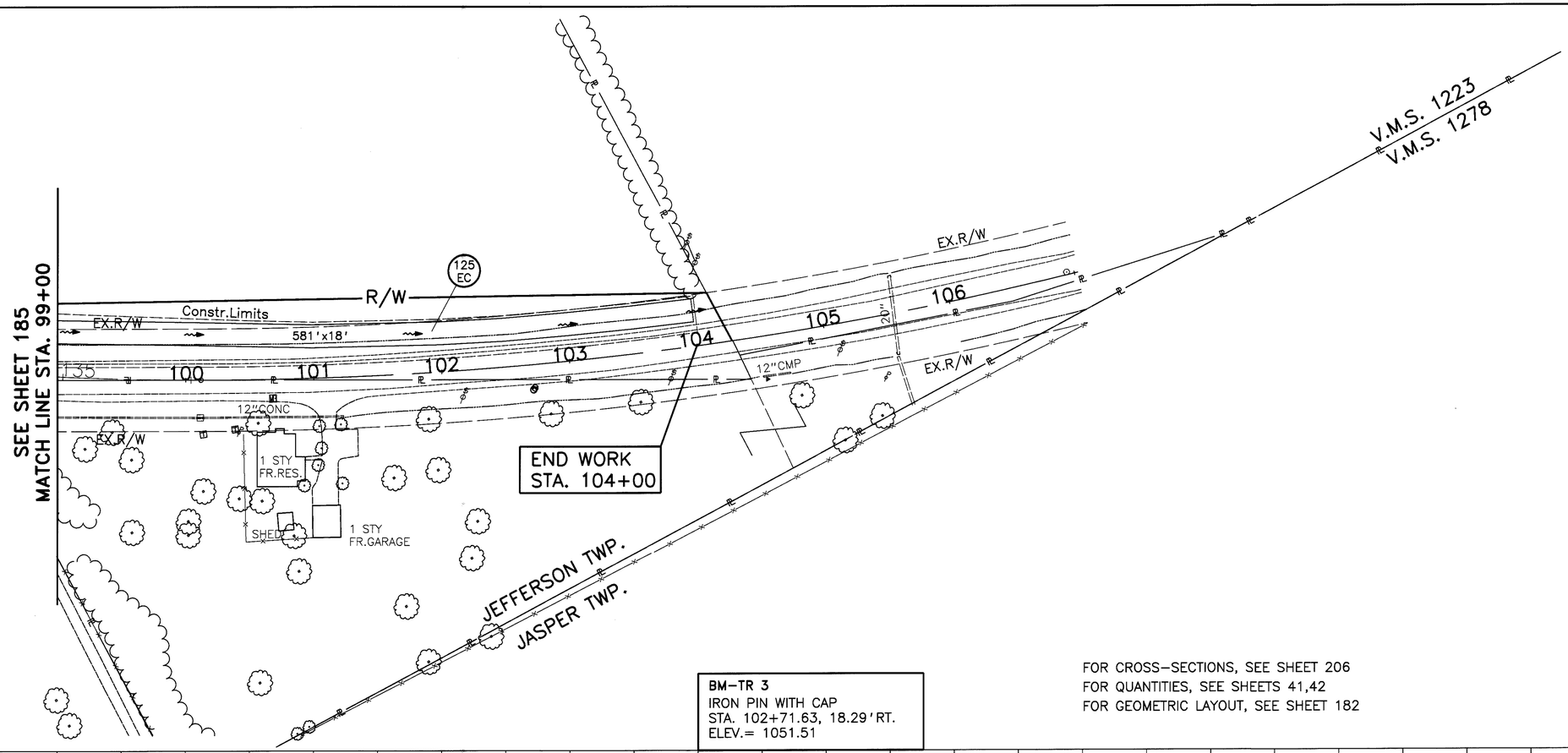
EXCAVATION	1844 C.Y.
EMBANKMENT	4370 C.Y.
SEEDING	13,409 S.Y.

HORIZONTAL SCALE IN FEET 0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE EX.U.S.R. 35
STA. 86 + 00 TO STA. 99 + 00

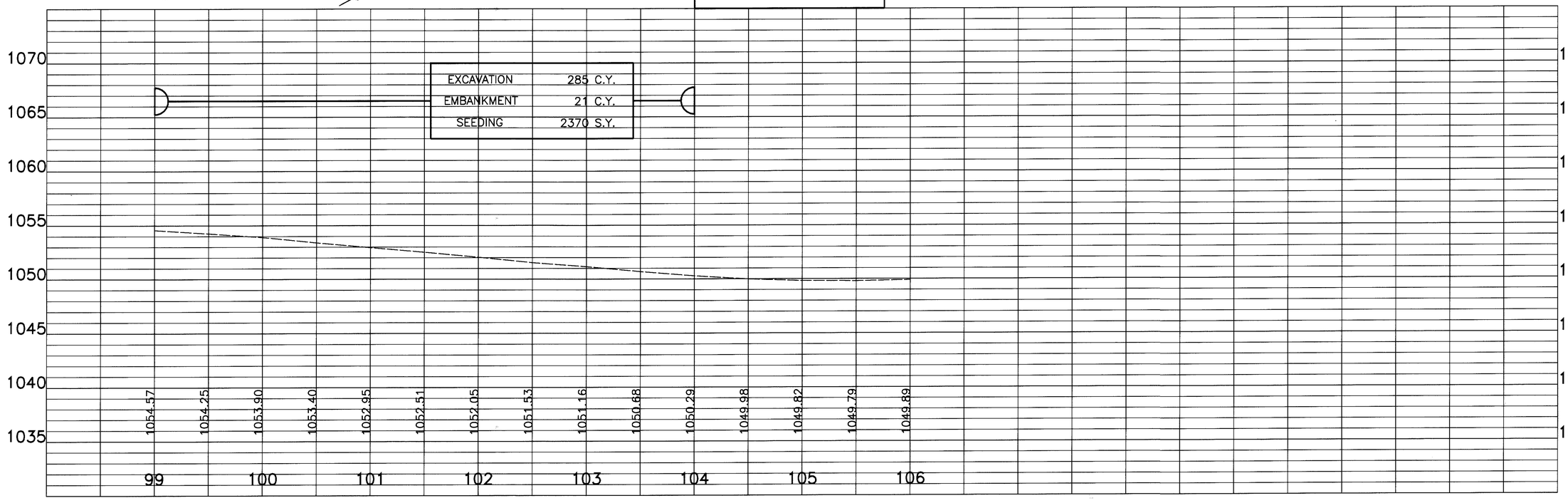
FAY-35-2.57



BM–TR 3
IRON PIN WITH CAP
STA. 102+71.63, 18.29'RT.
ELEV.= 1051.51

FOR CROSS–SECTIONS, SEE SHEET 206
FOR QUANTITIES, SEE SHEETS 41,42
FOR GEOMETRIC LAYOUT, SEE SHEET 182

EXCAVATION	285 C.Y.
EMBANKMENT	21 C.Y.
SEEDING	2370 S.Y.



HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED | CHECKED

PLAN & PROFILE EX.U.S.R. 35
STA. 99 + 00 TO STA. 106 + 00

FAY–35–2.57

186
522

0 30 60 120 HORIZONTAL SCALE IN FEET

CALCULATED | CHECKED

CROSS-SECTION LAYOUT

FAY-35-2.57

SEEDING END WIDTH	SEEDING SQ. YDS.
121	670
120	664
119	331
119	339
125	

(9-D) STA 63+61.123' LT.
INLET B" COND. TYPE 3
℄ = MEET EXISTING

(9-D) STA 63+95, 73.2 RT.
OUTLET B"CONDUIT TYPE B
℄ MEET EXISTING

STATION 63+50.00
1061.04
1060.58
36.16'
EX.12"
UGT
1056.1
64.66'
1056.2
52.84'

STATION 63+00.00
1060.78
1060.50
37.30'
1056.9
61.89'
1056.4
51.00'

STATION 62+50.00
1060.52
1060.44
38.43'
24'
1057.5
59.12'
1056.6
49.16'

STATION 62+25.00
1060.39
1060.39
39'
24'
1057.82
57.84'
1056.7
48.24'

+25 BEGIN NEW PAVEMENT

STATION 62+00.00
1060.26
EX. 1058.14
EX. 1058.20
UGT
BEGIN WORK +00

REMOVE EXISTING CONCRETE BASE UNDER ITEM 202

REMOVE EXISTING PAVEMENT UNDER ITEM 203 AND BACKFILL

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
92	49	184	82
107	40	231	55
142	19	137	14
AHEAD 154 / BACK 40	12 / 6	19	3
0	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

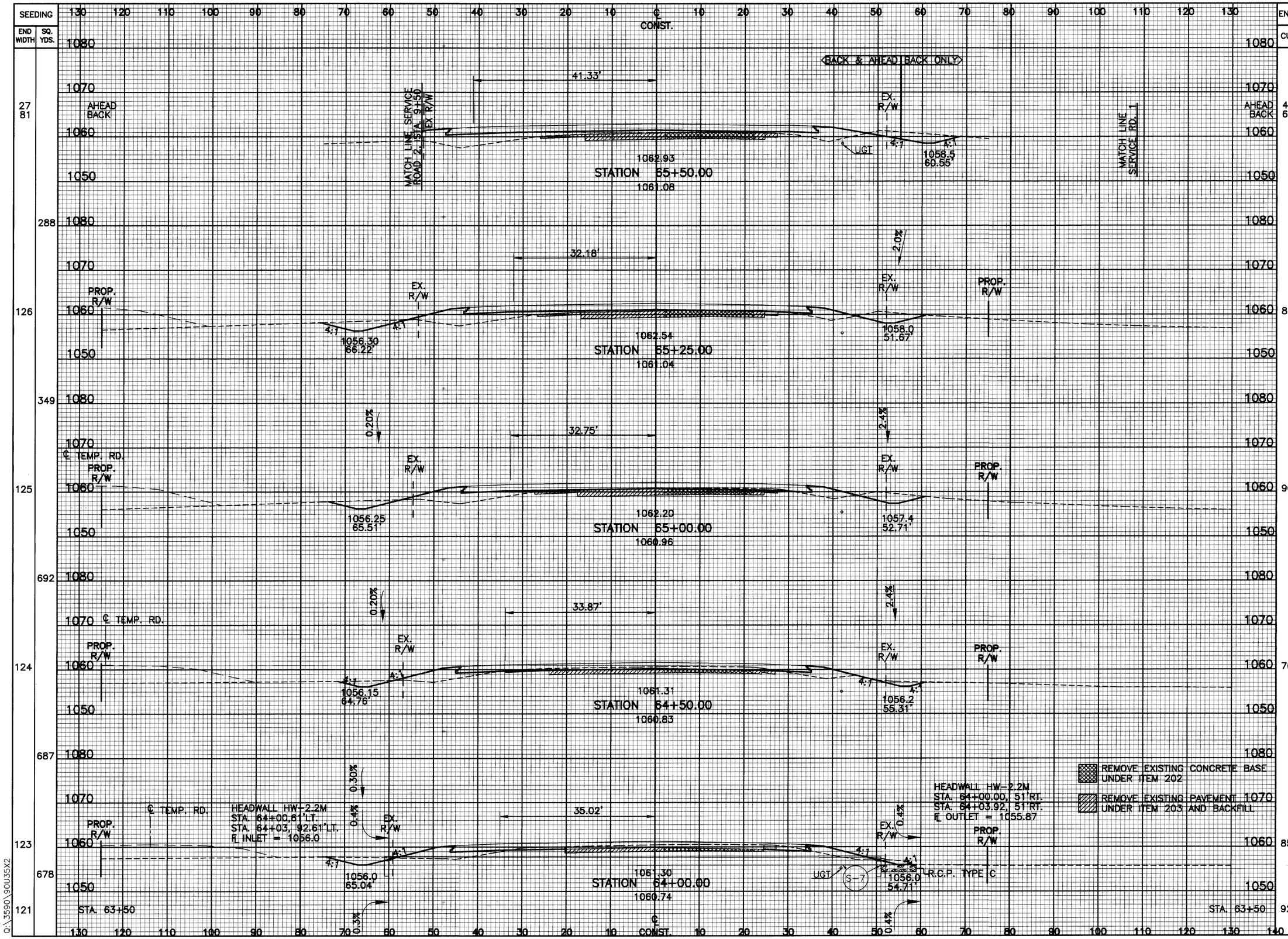
EXISTING U.S. 35 X-SECTIONS STATION 62+00 TO STATION 63+50

FAY-35-2.57

188
522

Q:\3590\90U35X1

SEEDING				END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.			CUT	FILL	CUT	FILL	CHECKED W.G.Y.
27		AHEAD		45	188			
81		BACK		65	188			
	288		STATION 65+50.00			69	159	
126			STATION 65+25.00	84	155			
	349		STATION 65+00.00			81	134	
125			STATION 65+00.00	90	134			
	692		STATION 64+50.00			154	215	
124			STATION 64+50.00	76	98			
	687		STATION 64+00.00			149	168	
123			STATION 64+00.00	85	83			
	678		STATION 64+00.00			164	122	
121		STA. 63+50		92	49			



EXISTING U.S. 35 X-SECTIONS STATION 64+00 TO STATION 65+50

FAY-35-2.57

189
522

Q:\3590\90U35X2

SEEDING END WIDTH	SEEDING SQ. YDS.
140	775
139	AHEAD
127	BACK
	267
65	AHEAD
13	BACK
	19
	0
	0
	38
	27

STA. 65+50

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130 140

1080 1070 1060 1050

PROP LA R/W

0.24%

28.20'

EX. R/W

5' BENCH ELEV.1059

4:1

1057.5 72.3'

1066.26

STATION 67+00.00

1061.32

0.35%

EX. R/W

PROP. LA R/W

1058.8 63.4'

UGT

0.24%

29.34'

AHEAD BACK

EX. R/W

1057.6 67.96'

1064.96

STATION 66+50.00

1061.23

BACK & AHEAD AHEAD ONLY

0.8%

MATCH LINE SERVICE RD. 1

EX. R/W

PROP. LA R/W

1058.4 59.76'

AHEAD ONLY BACK & AHEAD

0.24%

29.91'

AHEAD BACK

5' BENCH ELEV.1059

EX. R/W

1057.6 66.14'

1064.38

STATION 66+25.00

1061.19

MATCH LINE SERVICE ROAD 2, STA. 9+00

MATCH LINE SERVICE ROAD 1, STA. 10+50

EX. R/W

MATCH LINE SERVICE ROAD 2, STA. 9+50

1063.85

STATION 66+00.00

1061.17

MATCH LINE SERVICE ROAD 1, STA. 10+50

EX. R/W

MATCH LINE SERVICE ROAD 2, STA. 9+50

1063.37

STATION 65+75.00

1061.13

MATCH LINE SERVICE ROAD 1, STA. 10+50

UGT

EX. R/W

REMOVE EXISTING CONCRETE BASE UNDER ITEM 202

REMOVE EXISTING PAVEMENT UNDER ITEM 203 AND BACKFILL

STA. 65+50

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
9	500	52	842
AHEAD 47 / BACK 45	409 / 409	39	343
AHEAD 39 / BACK 34	331 / 286	32	241
35	234	33	206
36	210	38	184
45	188		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 65+75 TO STATION 67+00

FAY-35-2.57

190
522

Q:\3590\90U35X3

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.C.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
181	978	50	1459	77	2575	
159	853	21	1127	40	1793	
148	809	22	809	31	1334	
143	787	11	632	19	1048	
140		9	500			

EXISTING U.S. 35 X-SECTIONS STATION 67+50 TO STATION 69+50

FAY-35-2.57

191
522

Q:\3590\90U35X4

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
200	557	19	2836	15	2516
201	1114	13	2599	37	4476
200	1100	26	2235	64	3832
196	1070	43	1904	84	3301
189	1028	47	1661	90	2889
181		50	1459		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 70+00 TO STATION 71+75

FAY-35-2.57

192
522

CONST. ℄ BL RAMP C

STATION 71+75.00
1082.05
1062.36
PROP. LA R/W
EX. R/W
3.12%
2:1
2:1 NORM.
4:1
48:1
1059.2
93.0'
1056.2
117.25'
0.2%

STATION 71+50.00
1081.42
1062.26
1058.7
92.8'
1056.3
111.0'
UCT
2.0%

STATION 71+00.00
1080.04
1062.12
1057.7
92.0'
1056.4
101.6'
0.4%

STATION 70+50.00
1078.54
1062.07
5'BENCH ELEV.1059
1057.5
89.4'
1056.5
94.0'

STATION 70+00.00
1076.91
1062.03
1057.3
86.6'
1056.6
86.1'
STA. 69+50

℄ CONST. BL RAMP C

Q:\3590\90U35X5

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
107		73+50.00	19	2446		
	595				35	4267
107		73+00.00	19	2162		
	595				34	3739
107		72+50.00	18	1876		
	606				32	3314
111 205		72+00.00 AHEAD	16	1703		
		72+00.00 BACK	17	2972		
	563				17	2689
200		STA. 71+75	19	2836		

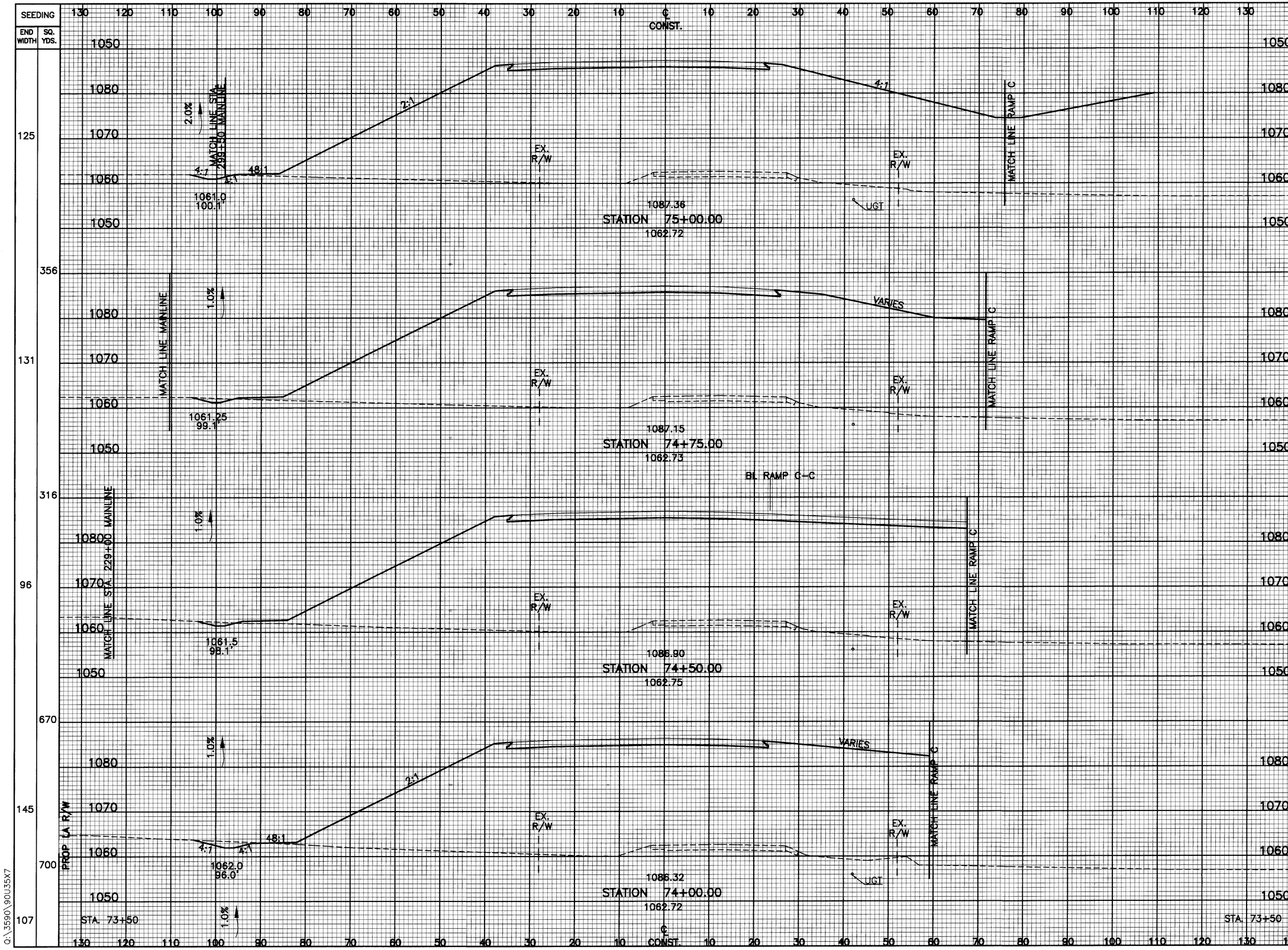
CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 72+00 TO STATION 73+50

FAY-35-2.57

193
522

Q:\3590\90U35X6



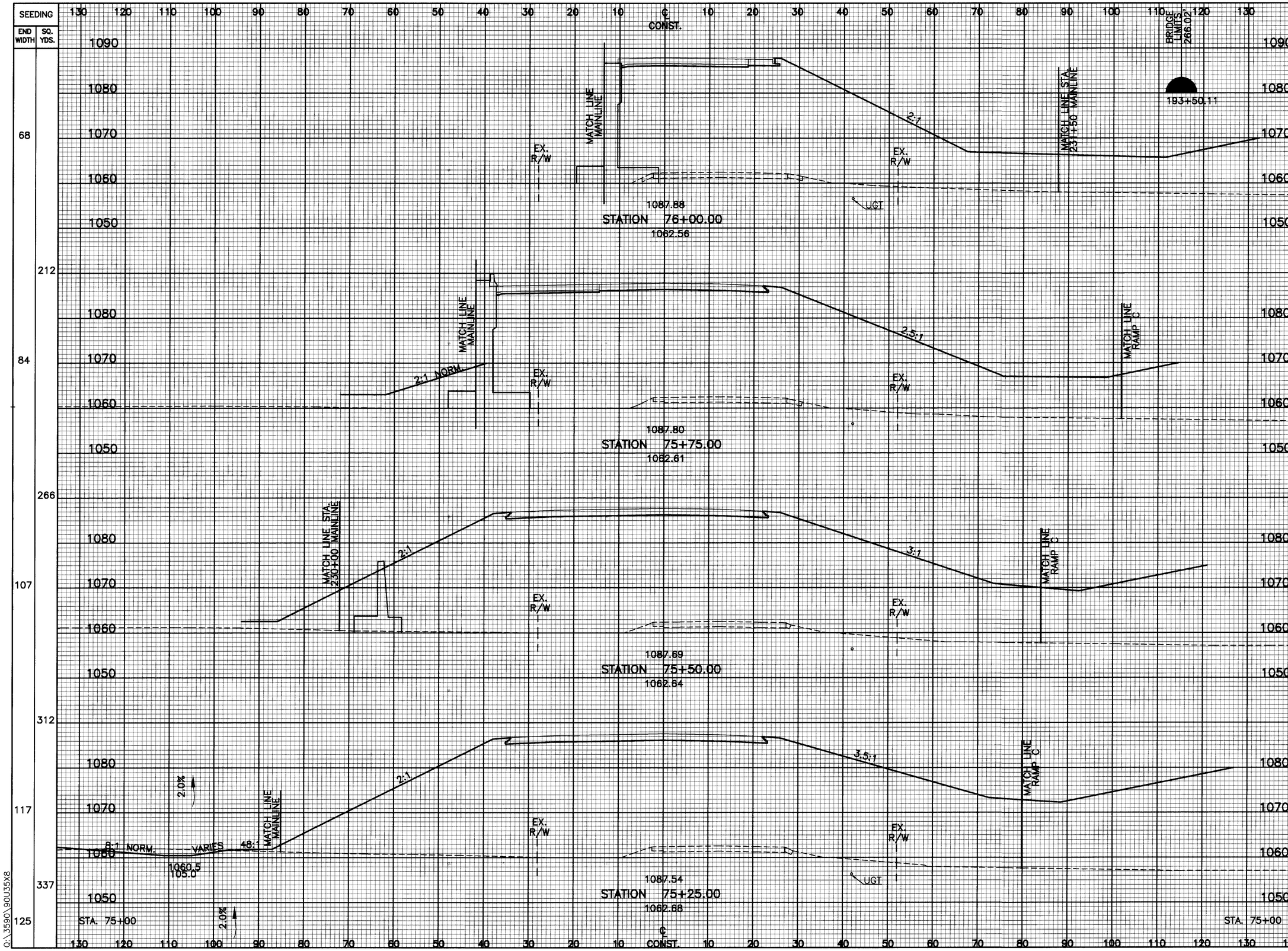
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
1	3260		
		3	3010
5	3242		
		5	2966
5	3164		
		16	5561
12	2842		
		29	4896
19	2446		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 74+00 TO STATION 75+00

FAY-35-2.57

Q:\3590\90U35X7



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
0	1856		
		0	2163
0	2817		
		0	2813
0	3259		
		0	3041
0	3309		
		1	3041
1	3260		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 75+25 TO STATION 76+00

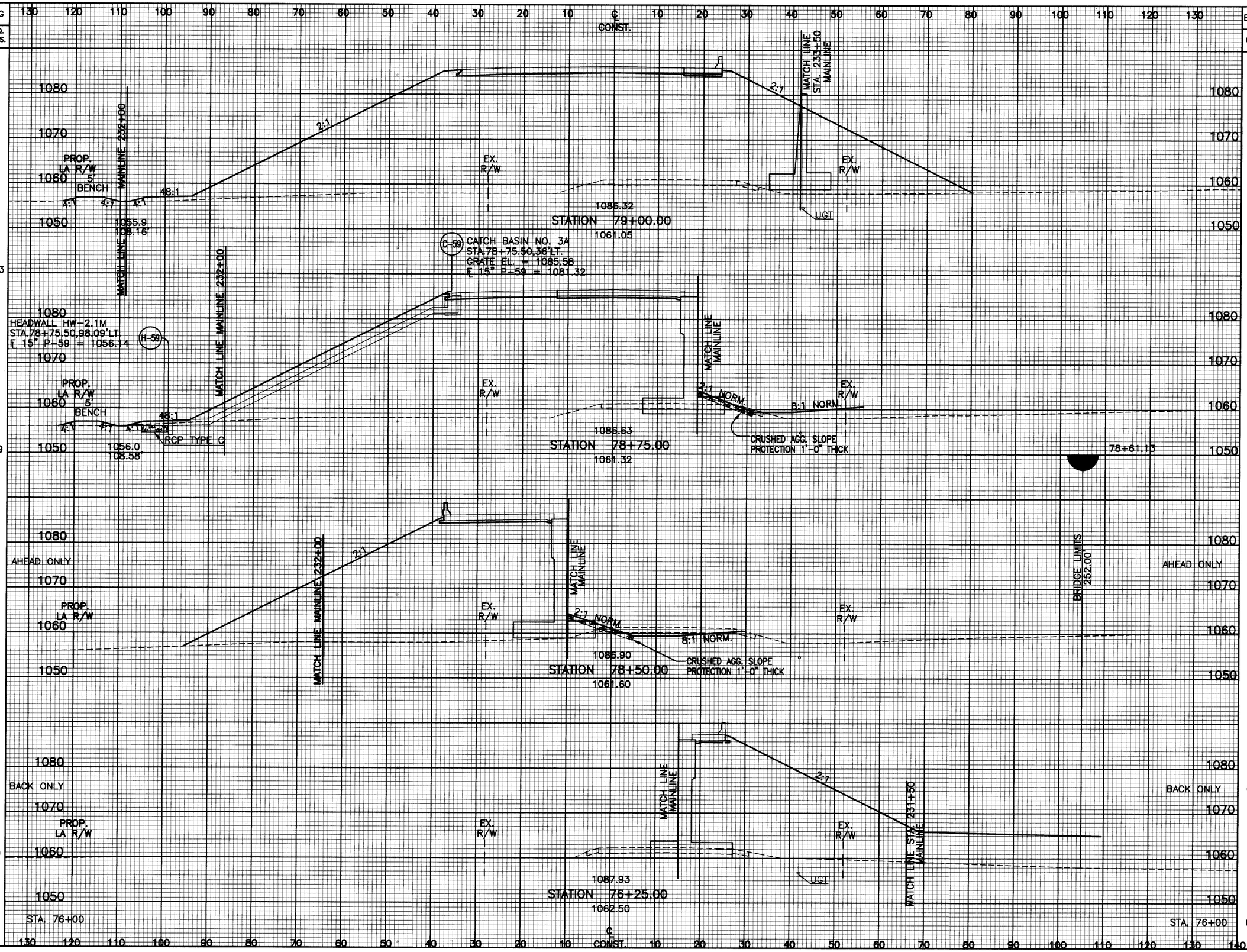
FAY-35-2.57

Q:\3590\90U35X8

SEEDING	
END WIDTH	SQ. YDS.
98	213
55	119
30	46
159	68

END AREA		VOLUME	
CUT	FILL	CUT	FILL
1	2736		
		1	2282
0	2193		
		0	1619
0	1305		
0	950		
		0	1299
0	1856		

CALCULATED J.L.G.
CHECKED W.G.Y.

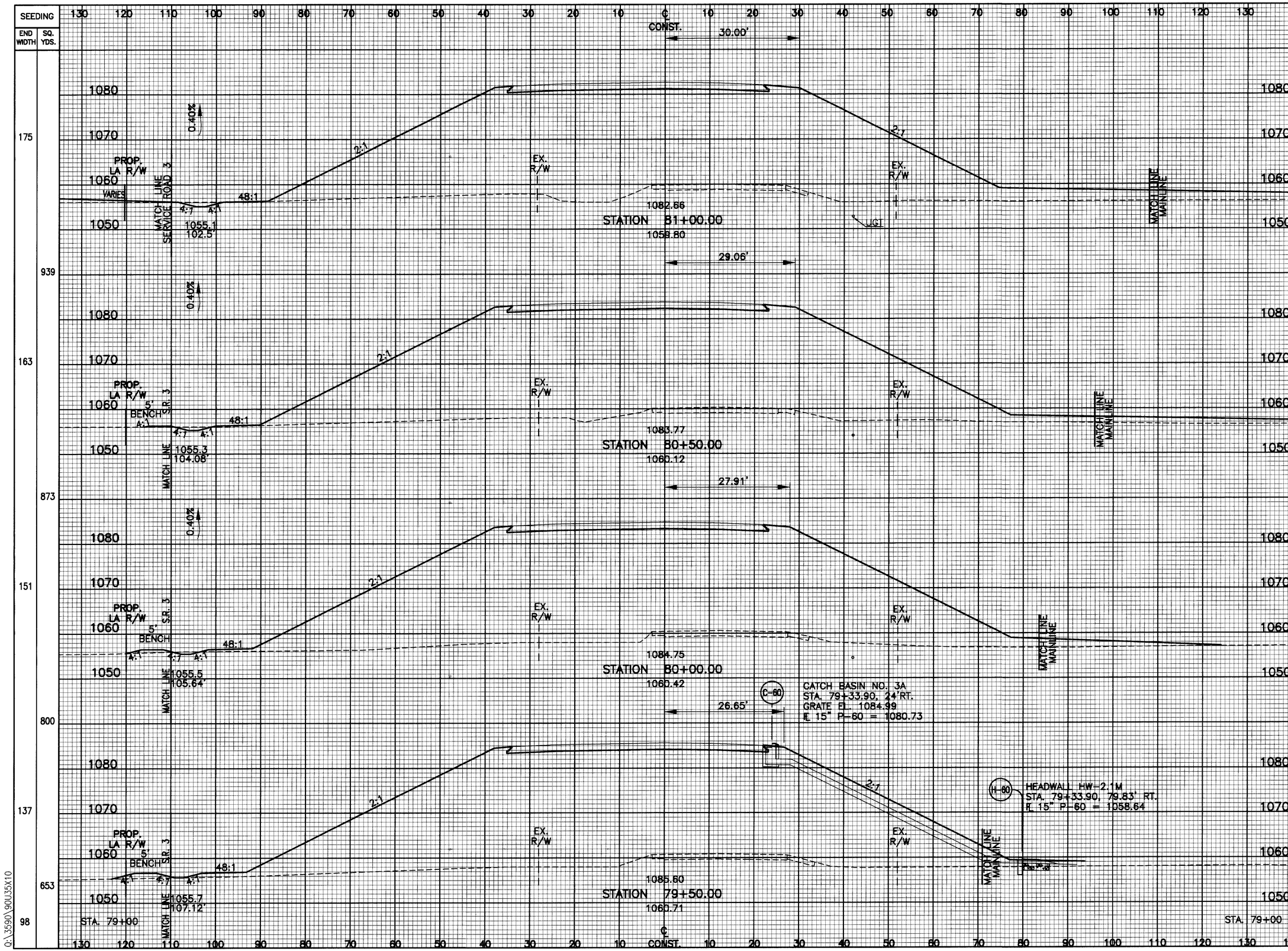


EXISTING U.S. 35 X-SECTIONS STATION 76+25 & STATIONS 78+50 TO 79+00

FAY-35-2.57

196
522

O:\3590\90U35X9



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
6	2845		
		11	5279
5	2856		
		8	5422
3	3000		
		3	5595
0	3043		
		0	5351
0	2736		

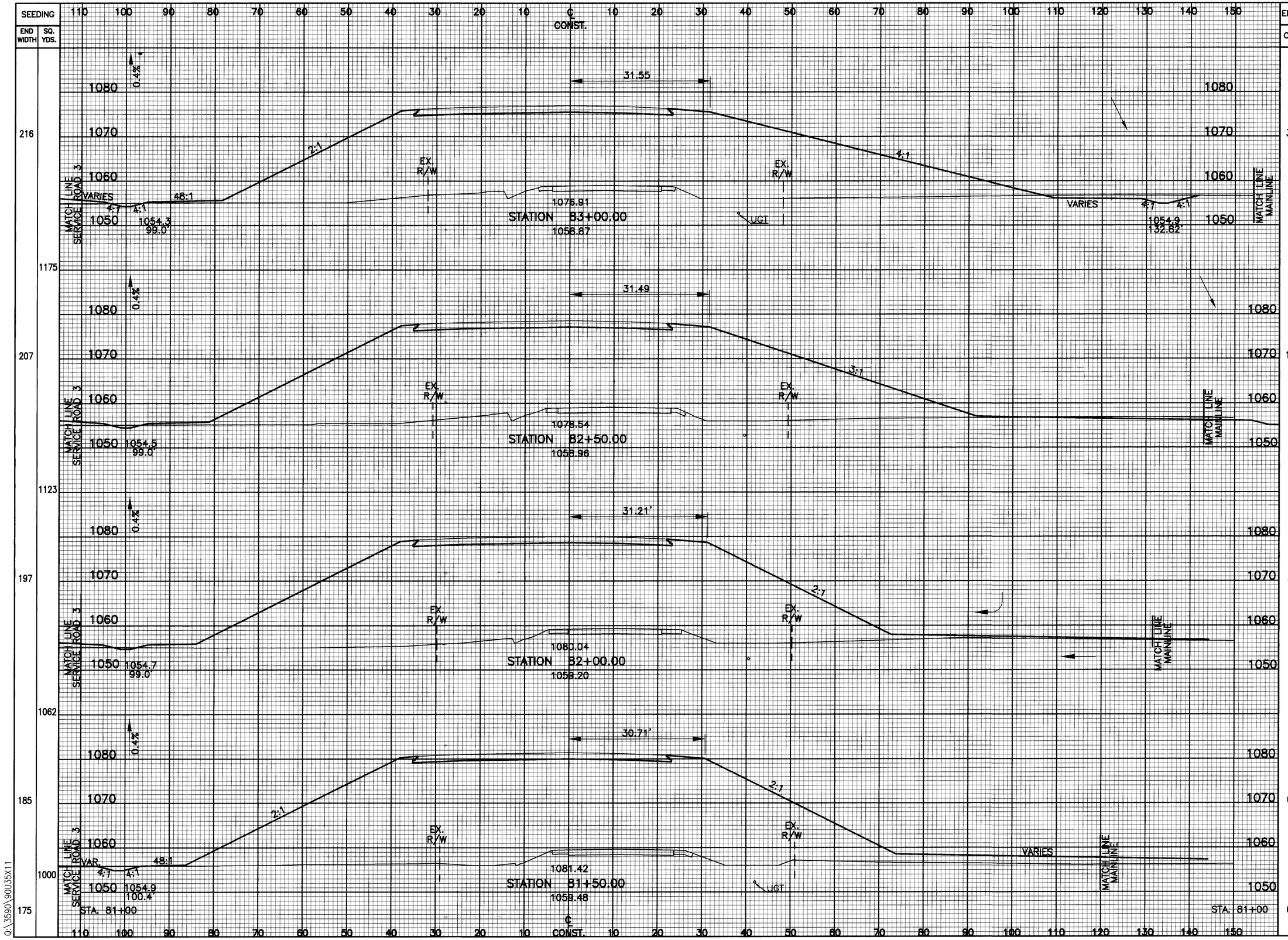
CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 79+50 TO STATION 81+00

FAY-35-2.57

Q:\3590\90U35X10

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
216		33	2386			
	1175			46	4506	
207		16	2481			
	1123			17	4653	
197		2	2544			
	1062			8	4869	
185		6	2715			
175		6	2845	12	5148	

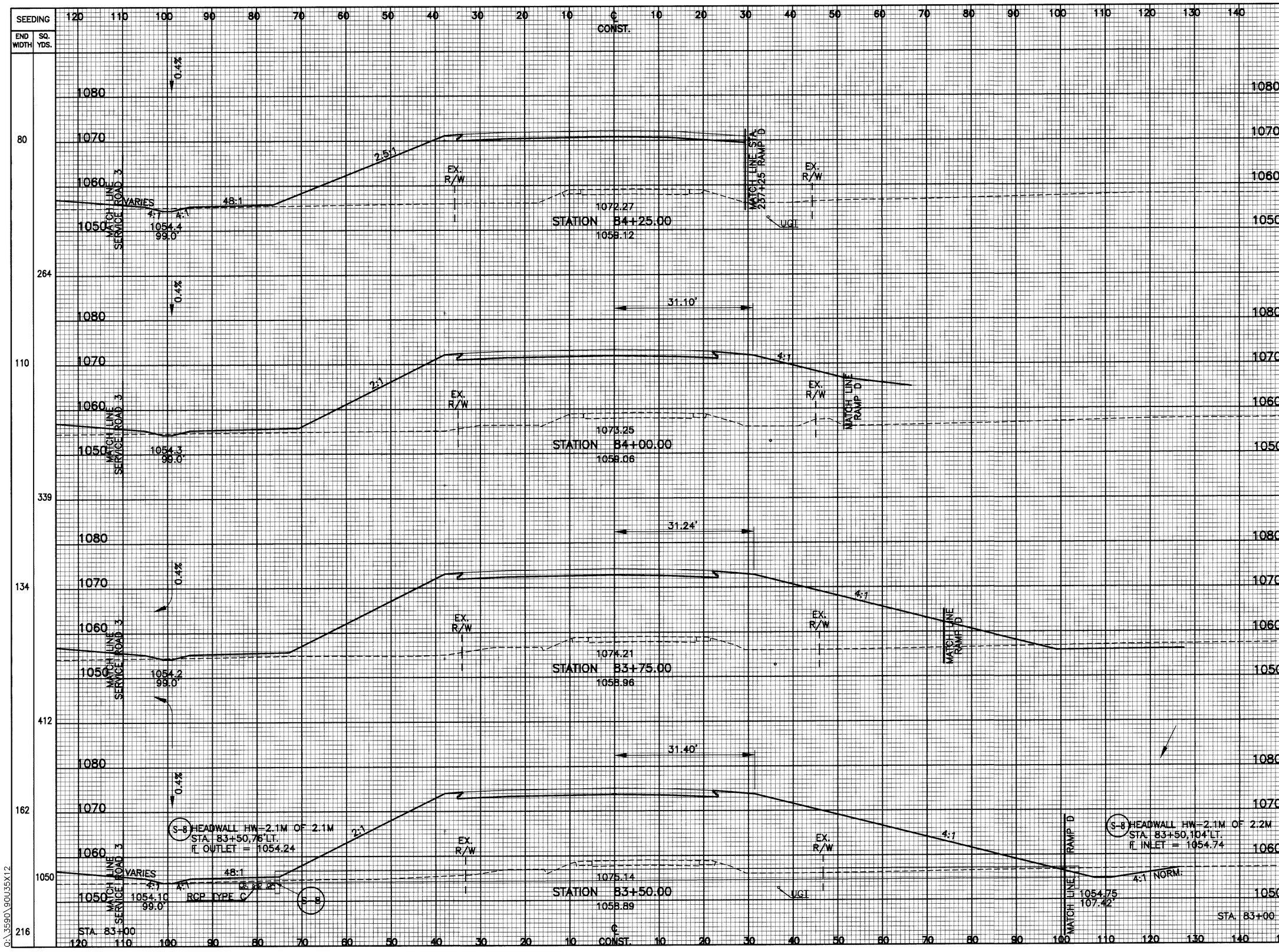


EXISTING U.S. 35 X-SECTIONS STATION 81+50 TO STATION 83+00

FAY-35-2.57

198
522

Q:\3590\90U35X11



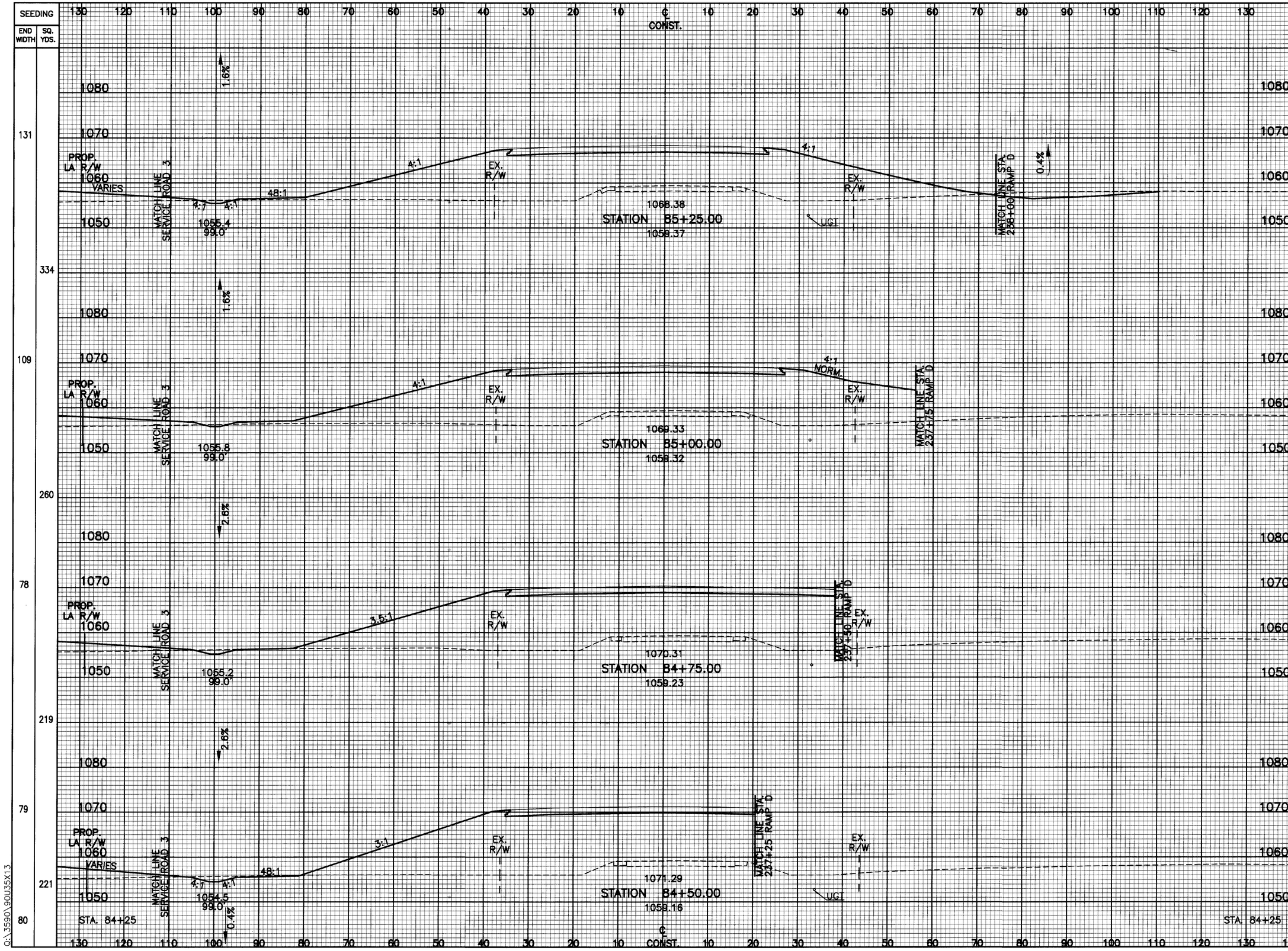
END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
4	1168		
		3	1272
2	1580		
		2	1600
1	1877		
		1	1839
1	2096		
		32	4150
33	2386		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 83+50 TO STATION 84+25

FAY-35-2.57

Q:\3590\90U35X12



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
4	1074		
		3	1059
2	1214		
		4	1085
6	1129		
		8	982
11	993		
		7	1000
4	1168		

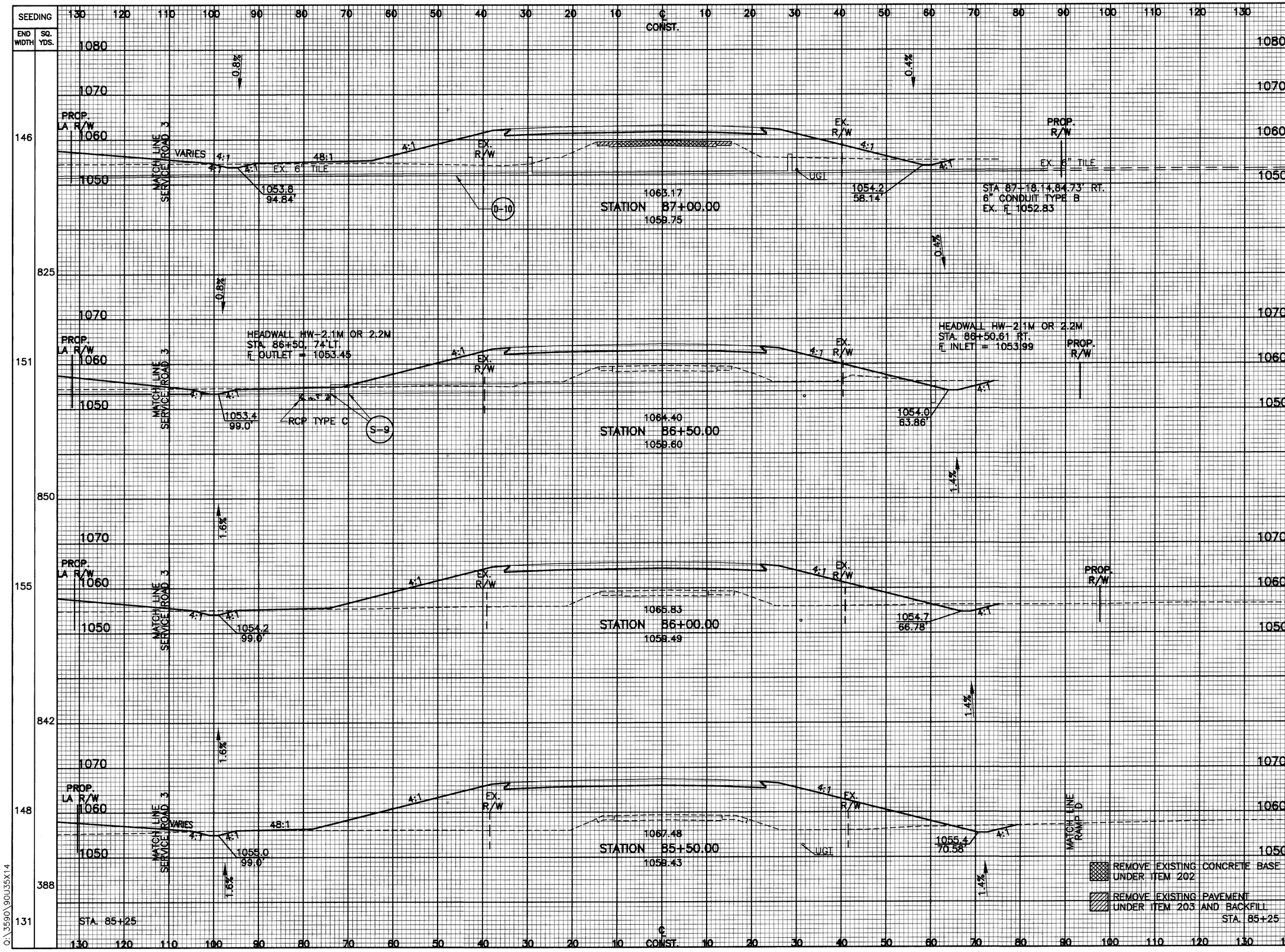
CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 84+50 TO STATION 85+25

FAY-35-2.57

200
522

Q:\3590\90U35X13



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
23	487		
		49	955
30	544		
		42	1169
15	719		
		31	1528
18	931		
		11	928
4	1074		

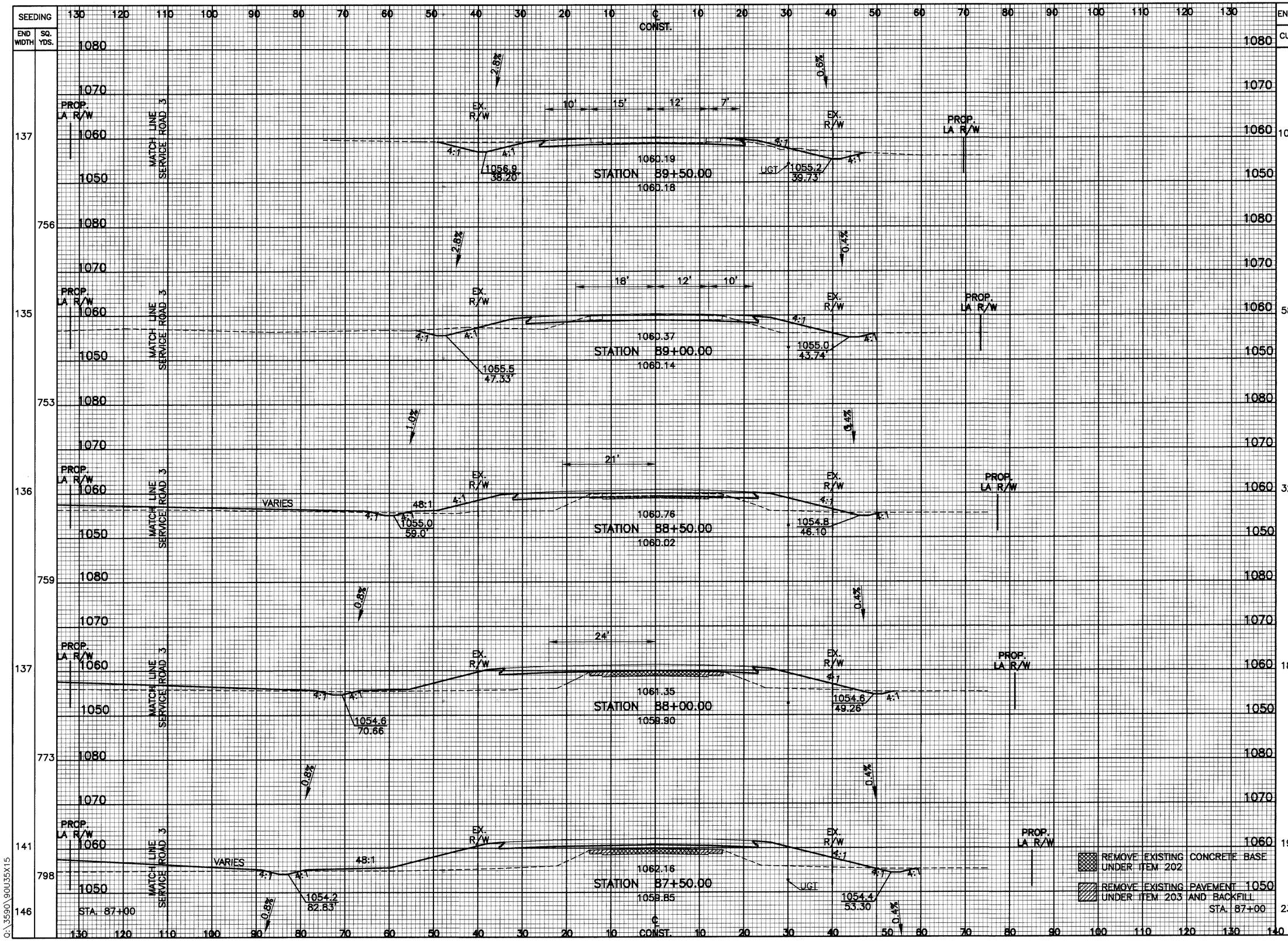
CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 85+50 TO STATION 87+00

FAY-35-2.57

Q:\3590\90U35X14

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
137	756	104	7	150	61	
135	753	58	59	83	217	
136	759	32	175	46	387	
137	773	18	243	34	552	
141	798	19	353	39	778	
146		23	487			

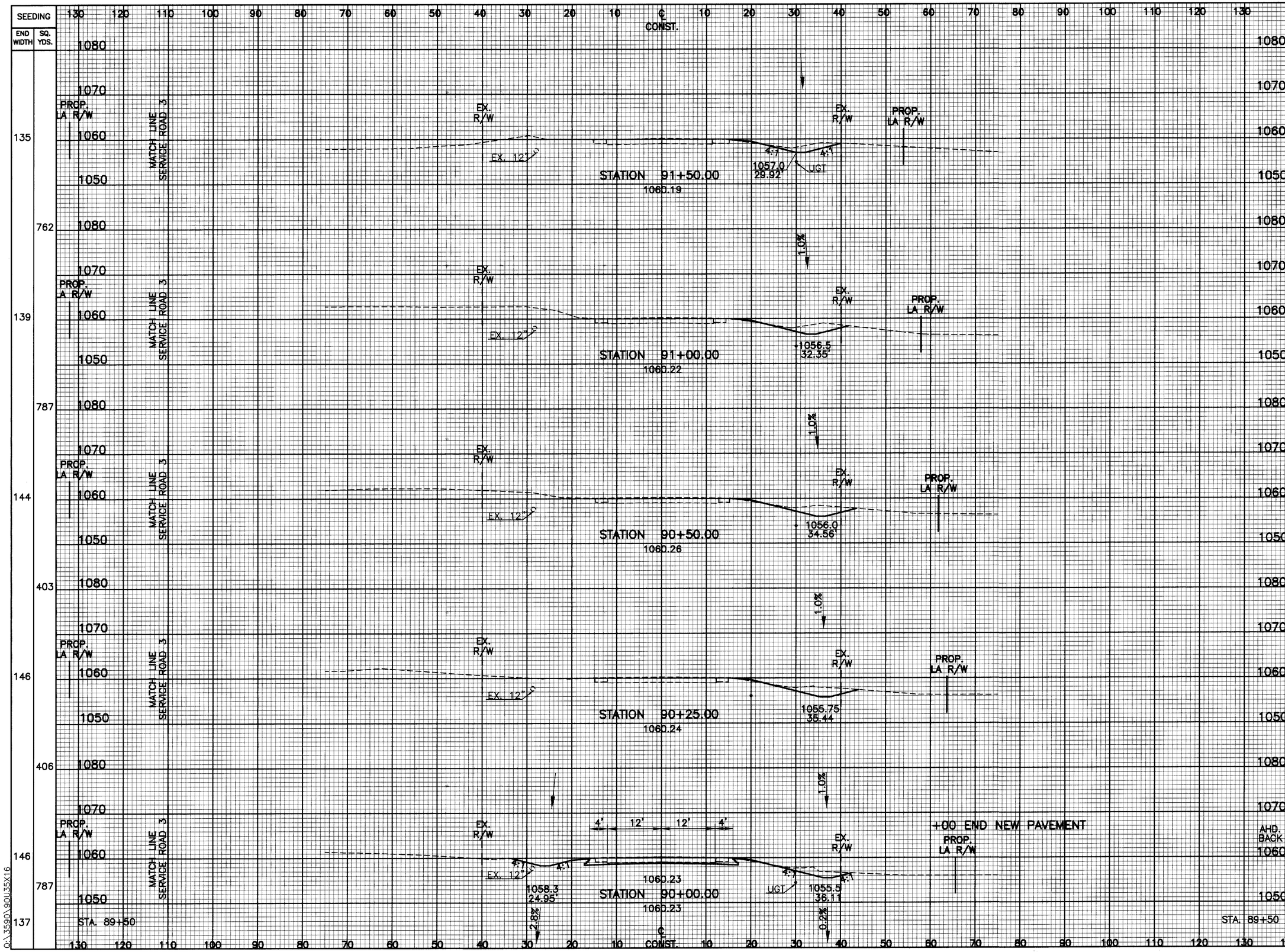


EXISTING U.S. 35 X-SECTIONS STATION 87+50 TO STATION 89+50

FAY-35-2.57

202
522

Q:\3590\90U35X15



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
14	2		
		31	3
19	1		
		38	3
22	2		
		20	2
22	2		
		24	2
AHD. 30	2		
BACK 78	2		
		169	8
104	7		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 90+00 TO STATION 91+50

FAY-35-2.57

Q:\3590\90U35X16

SEEDING END WIDTH	SEEDING SQ. YDS.
105	46
33	0
	0
	25
	18
	71
	33
	128
	59
	(-50)
	262
	129
	734
	135

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
71	21		
5	21		
		7	25
8	31		
		9	29
11	31		
		9	30
8	33		
		7	18
6	5		
		6	2
6	0		
0	0		
		(3)	(1)
0	0		
		13	2
14	2		

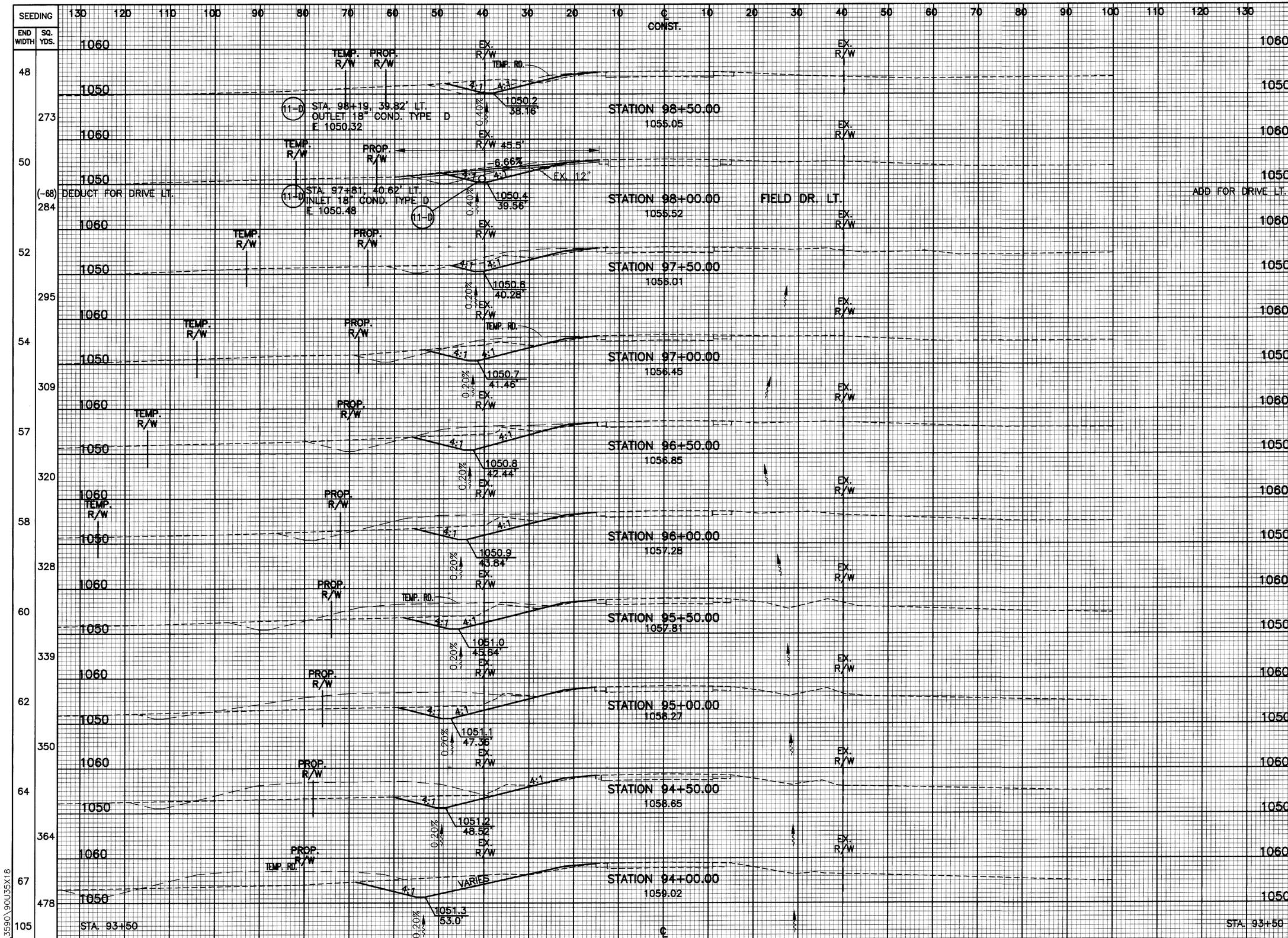
CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 92+00.00 TO STATION 93+50.00

FAY-35-2.57

204
522

Q:\3590\90U35X17



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
36	8		
		72	12
41	4		
		(4)	(1)
		64	9
28	5		
		75	11
52	6		
		106	10
62	4		
		108	8
54	4		
		106	8
60	4		
		103	8
51	4		
		89	9
45	5		
		125	11
89	6		
		149	25
71	21		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 94+00.00 TO STATION 98+50.00

FAY-35-2.57

Q:\3590\90U35X18

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
26	164	+00 END WORK	0	0	6	0
33	195	STATION 103+50.00	6	0	12	0
37	217	STATION 103+00.00	6	0	13	0
41	237	STATION 102+50.00	7	0	18	0
44	512	STATION 102+00.00	12	0	52	4
48	528	STATION 101+00.00	16	2	76	4
47	517	STATION 100+00.00	25	0	108	13
46	262	STATION 99+00.00	33	7	64	14
48		STA. 98+50	36	8		

CALCULATED J.L.G.
CHECKED W.G.Y.

EXISTING U.S. 35 X-SECTIONS STATION 99+00.00 TO STATION 104+00.00

FAY-35-2.57

206
522

Q:\3590\90U35X19

BM–TR 15
IRON PIN WITH CAP
STA. 71+84.05, 31.68' RT.EX.U.S.R.35
ELEV.= 1061.40

LA R/W
TEMP.R/W
EX.R/W
Constr.Limits
EX. U.S.R. 35
RAMP C
RAMP CC
CONSTR.
S.59°–22'–42.7"E
S.28°–13'–49.2"E
SHOULDER TAPER RT.
25:1
12' RT. STA. 74+57.81 EX. U.S.R. 35
P.C.STA. CC 229+01.04
P.T.STA. CC 230+41.01
20'LT. STA. C 230+59.33
P.T.STA. C 230+59.33
24'RT. STA. 69+87.48 EX. U.S.R. 35
P.C.STA. C 225+40.19
+48.46
P–24
S–7A
S–10
182.5'x8'
+27.5

CURVE DATA – RAMP CC
P.I.STA. 229+94.24 = 83.72'RT. STA. 73+98.30 EX. U.S.R. 35
P.C.STA. 229+01.04
P.T.STA. 230+41.01
Δ = 96°–37'–33.2"LT.
Dc= 69°–01"–51.8"
R = 83.00'
T = 93.20'
L = 139.97'
E = 41.80'

CURVE DATA – RAMP C
P.I.STA. 228+06.35
P.C.STA. 225+40.19
P.T.STA. 230+59.33
Δ = 31°–08'–53.5"RT.
Dc= 6°–00'
R = 954.93'
T = 266.16'
L = 519.14'
E = 36.40'
SE= .044 : 1

MATCH LINE STA. 234 + 00
SEE SHEET 208

FOR TYPICAL SECTIONS, SEE SHEET 7
FOR CROSS–SECTIONS, SEE SHEETS 209–212
FOR QUANTITIES, SEE SHEETS 38,42
FOR DRAINAGE DETAILS, SEE SHEETS 298,318
FOR UNDERDRAIN DETAILS, SEE SHEET 352
FOR PAVEMENT DETAILS, SEE SHEETS 268,269

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[E] ANCHOR ASSEMBLY, TYPE E–98

FOR EXISTING USR 35, SEE SHEETS 183,184

EXCAVATION	4 C.Y.
EMBANKMENT	51,428 C.Y.
SEEDING	10.049 S.Y.

PVI STA. 229+25
ELEV.= 1088.19
+3.20%
–2.80%
PVC STA. 225+50
ELEV.= 1076.19
750' V.C.
DESIGN SPEED = 50 mph
SSD = 408
PVI STA. 233+00
ELEV.= 1077.69

PVC STA. 229+50
ELEV.= 1085.06
PVI STA. 230+50
ELEV.= 1082.72
–3.56%
–1.12%
PVI STA. 229+25
ELEV.= 1083.28
100' V.C.
DESIGN SPEED = 55 mph
SSD = 457
UGT

RAMP C
RAMP CC

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

PLAN & PROFILE RAMP C
STA. 225 + 00 TO STA. 234 + 00

FAY–35–2.57

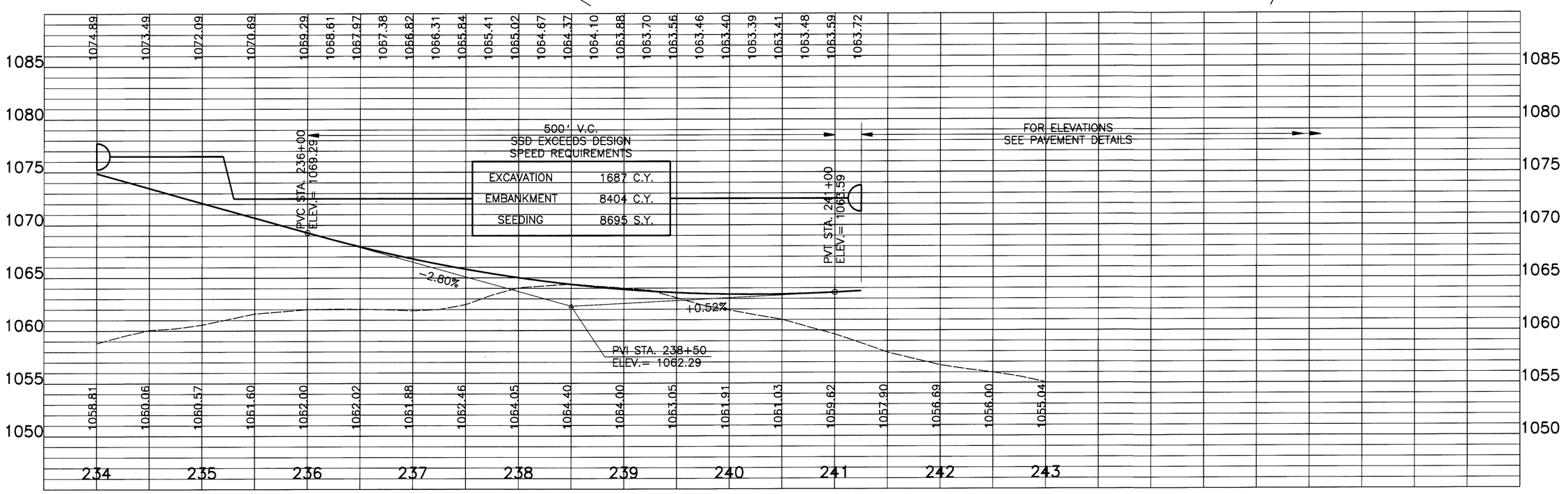
207
522

SEE SHEET 207
MATCH LINE STA. 234 + 00

55 EC
56 EC
P-25
235
236
237
238
239
240
241
242
243
244
245
246
PROP. U.S.R. 35
℄ CONSTR.
RAMP C
+19.33
16'
℄ CONSTR.
S.28°-13'-49.2"E
+50
6'
8'
16'
9'
29 GR
T
+10
182.5' x8'
Constr.Limits
LA R/W
126 EC
ELEC. EASEMENT
ELEC. EASEMENT
SHOULDER TAPER RT.
25:1
STA. C 243+00 =
83.25' RT. STA. 243+00 PROP U.S.R. 35
79' RT. STA. 246+00 PROP. U.S.R. 35
Constr.Limits
LA R/W

FOR TYPICAL SECTIONS, SEE SHEET 7
FOR CROSS-SECTIONS, SEE SHEETS 212-215
FOR QUANTITIES, SEE SHEETS 38,41,42
FOR PAVEMENT DETAILS, SEE SHEETS 269,270
FOR UNDERDRAIN DETAILS, SEE SHEET 352

T ANCHOR ASSEMBLY, TYPE T



0 25 50 100 HORIZONTAL SCALE IN FEET

CALCULATED
CHECKED

PLAN & PROFILE RAMP C
STA. 234 + 00 TO STA. 243 + 00

FAY-35-2.57

208
522

SEEDING	
END WIDTH	SQ. YDS.
104	536
89	497
90	506
92	

END AREA		VOLUME			
CUT	FILL	CUT	FILL	CALCULATED	J.L.G.
0	1906			CHECKED	W.G.Y.
	1630	0	3275		
	1433	0	2837		
	1269	1	2502		

MATCH LINE EX. U.S.R. 35

1081.4
39.7'

2:1

PROP. LA R/W

UGT

1082.49
STATION 229+00.00
1056.97

MATCH LINE EX. U.S.R. 35

6'

2:1

PROP. LA R/W

1082.19
STATION 228+50.00
1057.60

MATCH LINE EX. U.S.R. 35

7.94' 4.5'

2:1

PROP. LA R/W

1081.69
STATION 228+00.00
1058.00

MATCH LINE EX. U.S.R. 35 STA. 72+00

9.94' 4'

2:1

4:1

4:1

UGT

1080.99
STATION 227+50.00
1058.86

1056.2
73.56

PROP. LA R/W

FAY-35-2.57

209
522

Q:\3590\90RCX1

SEEDING	
END WIDTH	SQ. YDS.
172	466
163	806
127	756
145	692
104	

MATCH LINE EX. U.S.R. 35

0.030'/'

4:1

2:1

PROP. LA R/W

1081.97
STATION 230+75.00
1057.03

BL RAMP C C
0.037'/'

1082.19
STATION 230+50.00
1057.02

UGT

1082.49
STATION 230+00.00
1057.08

(C-24) CATCH BASIN NO 2-2B
STA 229+50, 41.6' LT
GRATE EL.=1081.00
℄ 15" P-24=1078.08

1081.2
41.6'

(H-24) HEADWALL HW-2.1N
STA 229+50, 61.07' RT
℄ 15" P-24=1056.78

1082.59
STATION 229+50.00
1056.91

RCP, TYPE C

STA. 229+00

END AREA		VOLUME		CALCULATED	J.L.G.
CUT	FILL	CUT	FILL	CHECKED	W.G.Y.
0	2903				
		0	2656		
0	2833				
		0	5066		
0	2638				
		0	4525		
0	2248				
		0	3847		
0	1906				

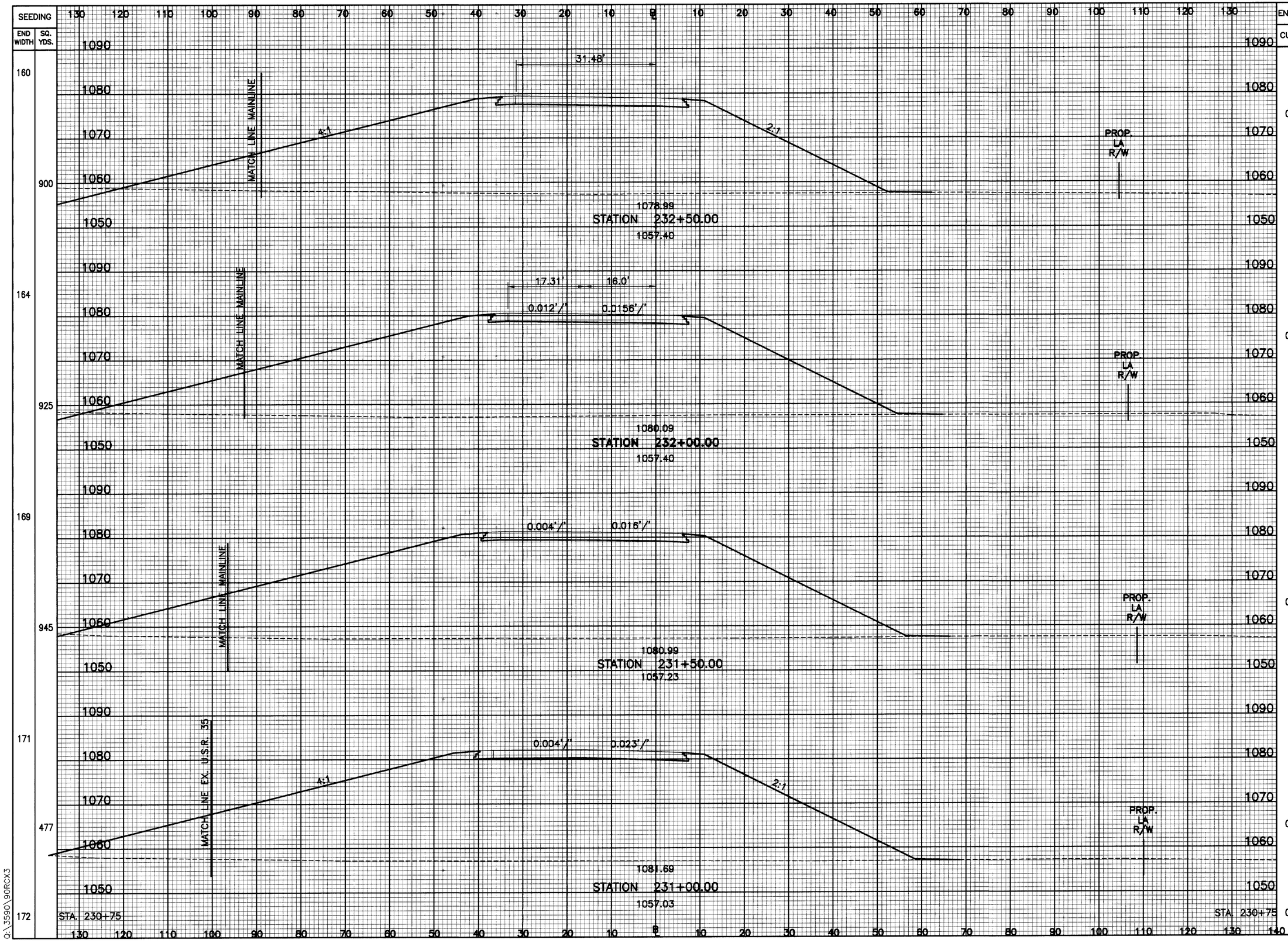
RAMP "C" X-SECTIONS STATION 229+50 TO STATION 230+75

FAY-35-2.57

210
522

Q:\3590\90RCX2

SEEDING END WIDTH	SEEDING SQ. YDS.
160	900
164	925
169	945
171	477
172	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
0	2199	0	4298
0	2442	0	4708
0	2642	0	5080
0	2844	0	2661
0	2903		

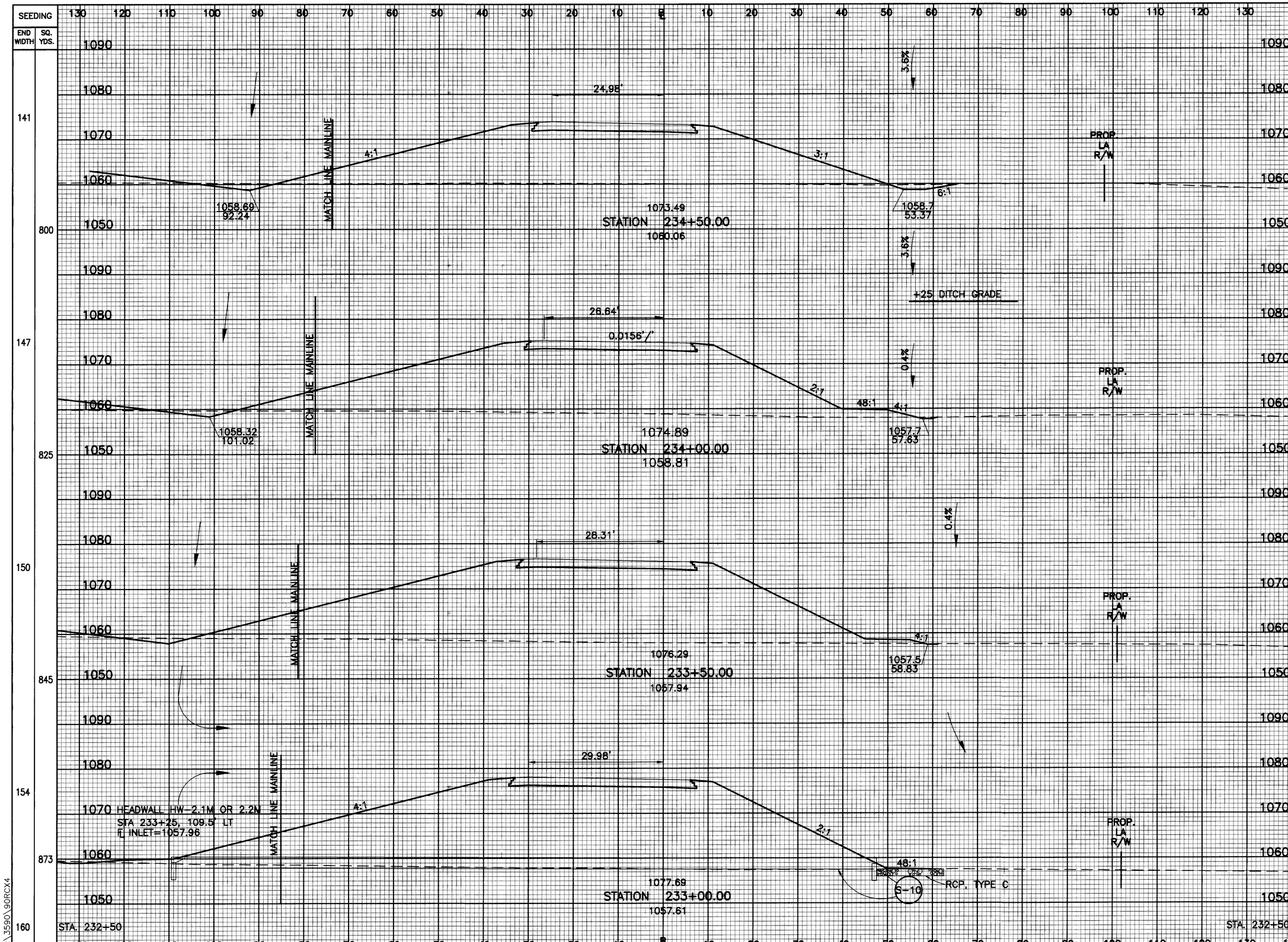
CALCULATED J.L.G.
CHECKED W.G.Y.

RAMP "C" X-SECTIONS STATION 231+00 TO STATION 232+50

FAY-35-2.57

211
522

Q:\3590\90RCX3



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
10	1115		
		11	2230
1	1368		
		2	2810
1	1666		
		1	3335
0	1935		
		0	3828
0	2199		

CALCULATED J.L.G.
CHECKED W.G.Y.

RAMP "C" X-SECTIONS STATION 233+00 TO STATION 234+50

FAY-35-2.57

Q:\3590\90RCX4

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
123	695	21	331	46	728	
127	720	38	455	71	1009	
132	748	38	634	51	1409	
137	773	17	887	25	1854	
141		10	1115			

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120 130

1090 1080 1070 1060 1050

MATCH LINE MAINLINE

18.31'
4:1
4:1
6:1
0.4%
1059.74
56.88
1067.97
STATION 236+50.00
1062.02
PROP. LA R/W
1061.1
35.81

19.98'
4:1
6:1
0.4%
1059.49
66.94
1069.29
STATION 236+00.00
1062.00
PROP. LA R/W
1060.9
41.89

21.64'
4:1
6:1
0.4%
1059.26
1070.69
STATION 235+50.00
1061.60
PROP. LA R/W
1060.7
48.29
+25 DITCH GRADE

23.31'
4:1
4:1
6:1
3.6%
1058.99
85.66
1072.09
STATION 235+00.00
1060.57
PROP. LA R/W
1060.5
54.69
3.6%

STA. 234+50
STA. 234+50

RAMP "C" X-SECTIONS STATION 235+00 TO STATION 236+50

FAY-35-2.57

213
522

Q:\3590\90RCX5

SEEDING END WIDTH	SEEDING SQ. YDS.	STATION	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
95	545	STATION 239+00.00	156	0	258	2
101	575	STATION 238+50.00	122	2	203	13
106	623	STATION 238+00.00	97	11	115	124
118	656	STATION 237+50.00	27	122	37	315
118	670	STATION 237+00.00	12	218	31	509
123		STA. 236+50	21	331		

CALCULATED J.L.G.
CHECKED W.G.Y.

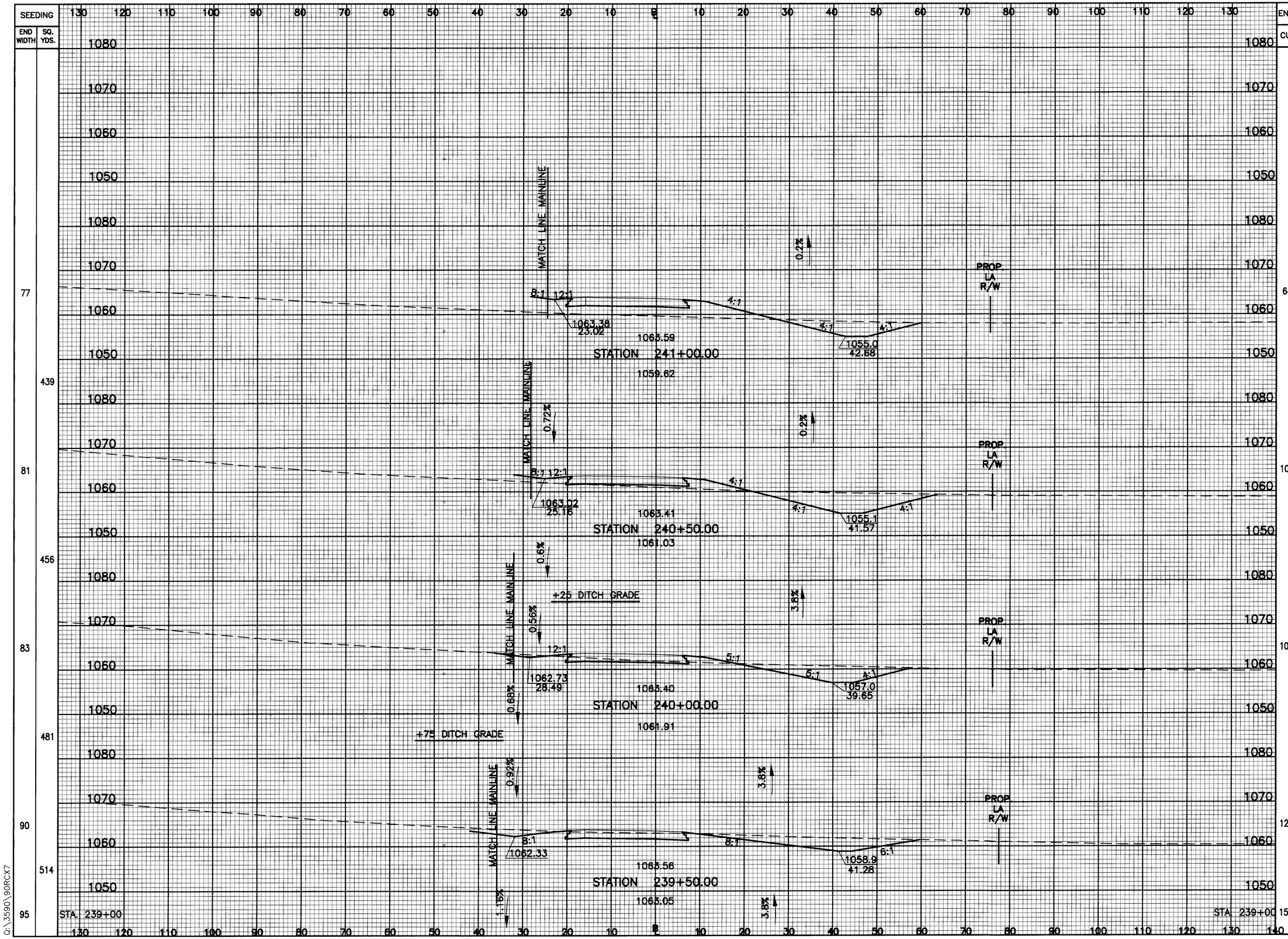
RAMP "C" X-SECTIONS STATION 237+00 TO STATION 239+00

FAY-35-2.57

214
522

Q:\3590\90RCX6

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
77	439	64	115	161	145	
81	456	109	41	198	50	
83	481	104	13	216	14	
90	514	129	2	264	2	
95	STA. 239+00	156	0	STA. 239+00		



RAMP "C" X-SECTIONS STATION 239+50 TO STATION 241+00

FAY-35-2.57

215
522

Q:\3590\90RCX7

BM–TR 221
IRON PIN WITH CAP
STA. 248+78.96, 106.87'LT.
ELEV.= 1056.28

CURVE DATA
P.I.STA. 242+91.95
P.C.STA. 238+00.00
P.C.C.STA. 247+47.74
Δ = 37°–54'–35.2"LT.
Dc= 4°–00'
R = 1432.39'
T = 491.95'
L = 947.74'
E = 82.13'
SE= .051 : 1

CURVE DATA
P.I.STA. 250+25.81
P.C.C.STA. 247+47.74
P.T.STA. 253+00.00
Δ = 16°–34'–03.6"LT.
Dc= 3°–00'
R = 1909.86'
T = 278.07'
L = 552.26'
E = 552.26'
SE= .051 : 1

STA. D 236+98.70 =
STA. 84+29.06 EX U.S.R. 35

[E] ANCHOR ASSEMBLY, TYPE E

FOR TYPICAL SECTIONS, SEE SHEET 7
FOR CROSS–SECTIONS, SEE SHEETS 217–221
FOR QUANTITIES, SEE SHEET 42
FOR DRAINAGE DETAILS, SEE SHEET 299
FOR PAVEMENT DETAILS – SEE SHEETS 272,273
FOR UNDERDRAIN DETAILS, SEE SHEET 352
FOR EXISTING USR 35, SEE SHEET 184

EXCAVATION	1179 C.Y.
EMBANKMENT	18,383 C.Y.
SEEDING	7.727 S.Y.

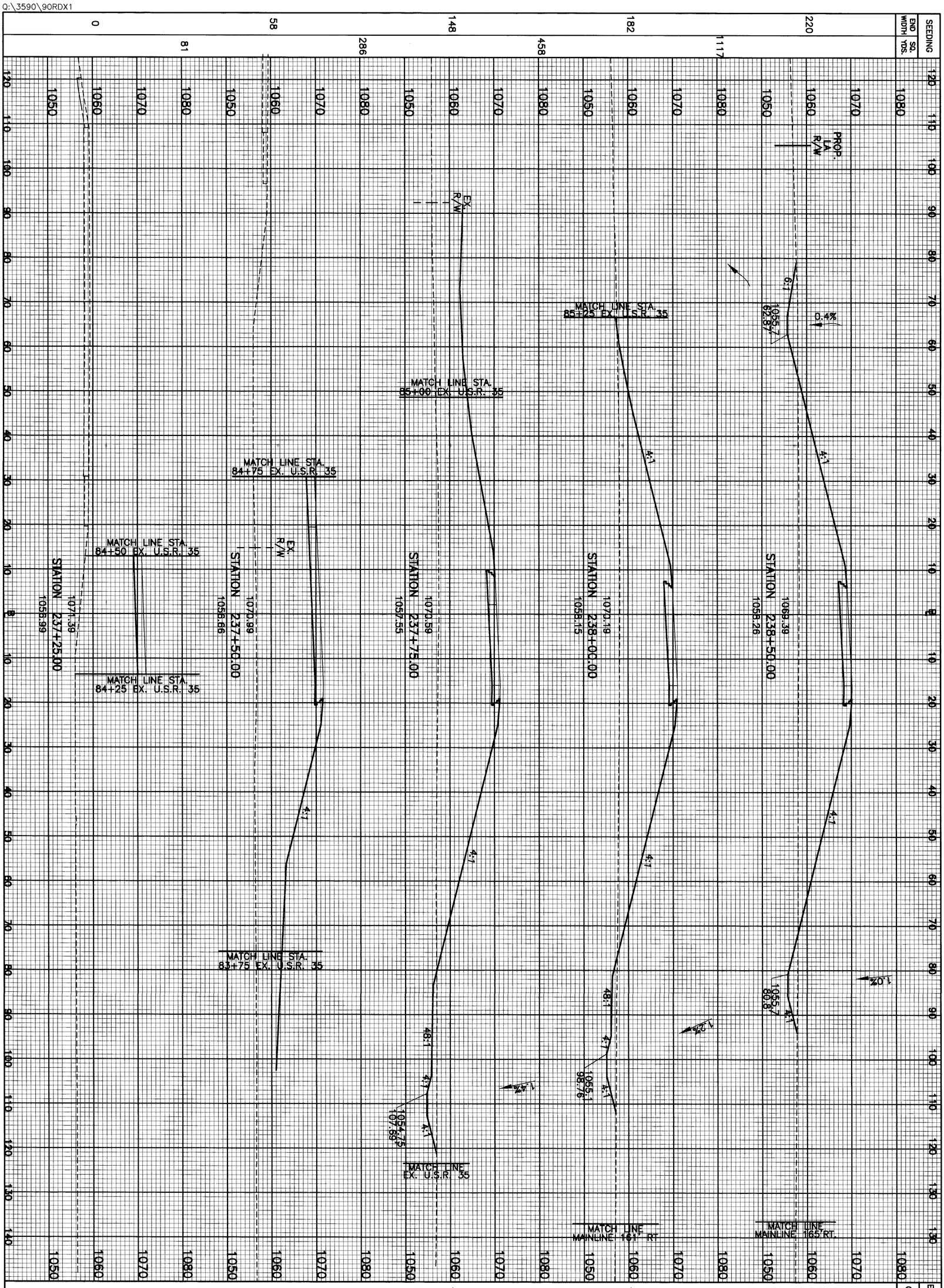
PVI STA. 237+25
ELEV.= 1071.39
E.P. +11.42
–1.60%
PVC STA. 241+50
ELEV.= 1064.59
600' V.C.
SSD EXCEEDS DESIGN SPEED REQUIREMENTS
–1.60%
+0.28%
PVI STA. 244+75
ELEV.= 1059.39
PVT STA. 248+00
ELEV.= 1060.30
FOR ELEVATIONS SEE PAVEMENT DETAILS
UGT
S–11

PLAN & PROFILE RAMP D
STA. 237 + 00 TO 250 + 00

FAY–35–2.57

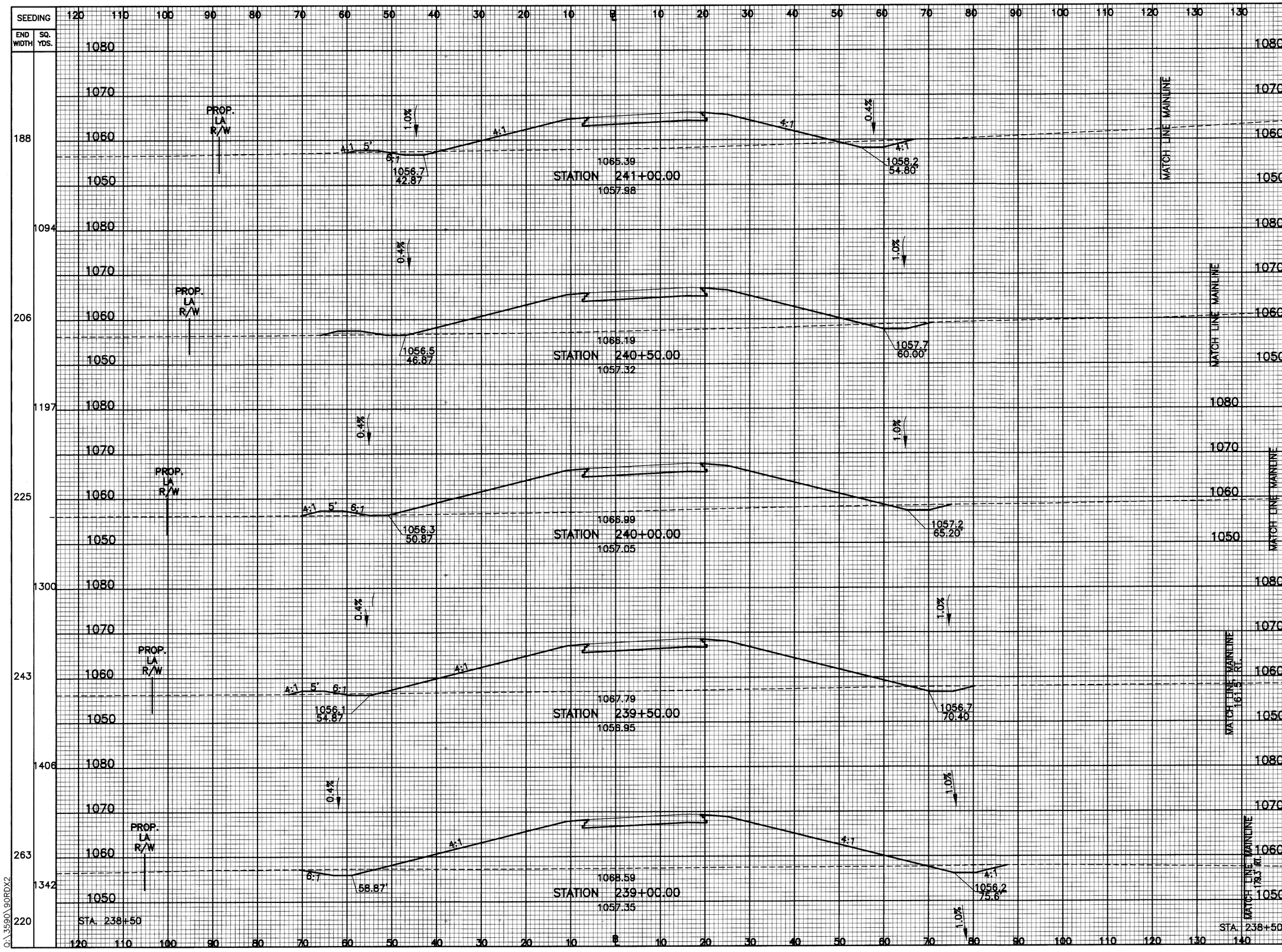
216
522

Q:\3590\90RDX1



217 / 522	FAY-35-2.57	RAMP "D" X-SECTIONS STATION 237+25 TO STATION 238+50

CALCULATED J.L.G.
CHECKED W.G.Y.



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
21	413		
		32	906
13	565		
		23	1169
11	697		
		24	1377
14	790		
		44	1513
33	844		
		90	1573
64	854		

CALCULATED J.L.G.
CHECKED W.G.Y.

RAMP "D" X-SECTIONS STATION 239+00 TO STATION 241+00

FAY-35-2.57

Q:\3590\90RDX2

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
136	783	43	290	66	508	
146	844	28	258	62	426	
158	881	38	202	82	375	
159	914	50	203	77	447	
170	994	33	279	50	641	
188		21	413			

HEADWALL HW-2.1M OR 2.2M
STA 243+50, 36.5' LT
℄ OUTLET=1054.41

HEADWALL HW-2.1M OR 2.2M
STA 243+50, 52.5' RT
℄ INLET= 1054.98

+75 DITCH GRADE

PROP. LA R/W

RCP, TYPE C

MATCH LINE MAINLINE

STATION 243+50.00
1061.97
1055.36
1054.4 38.39
1055.0 53.92
S-11

STATION 243+00.00
1062.52
1057.15
35.79'
1056.2 51.32

STATION 242+50.00
1063.14
1058.38
1056.4 35.07
1057.4 49.0

STATION 242+00.00
1063.83
1059.00
1056.8 38.23
1058.6 46.96

STATION 241+50.00
1064.59
1058.73
6:1
1057.2 37.67
1058.4 50.80

STA. 241+00

RAMP "D" X-SECTIONS STATION 241+50 TO STATION 243+50

FAY-35-2.57

219
522

Q:\3590\90RDX3

SEEDING	
END WIDTH	SQ. YDS.
82	483
92	539
102	597
113	656
123	719
136	

STA. 243+50

PROP. LA R/W

0.4%

6:1 4:1 8:1 8:1

MATCH LINE MAINLINE

1053.4 35.79 — 1060.32 — 1058.79 39.3

STATION 246+00.00

1054.33

1053.6 45.71 — 1060.50 — 1058.15 45.86

STATION 245+50.00

1054.45

1053.8 35.95 — 1060.76 — 1057.51 53.11

STATION 245+00.00

1054.41

1054.0 36.47 — 1061.09 — 1056.75 61.76

STATION 244+50.00

1054.53

1054.2 37.27 — 1061.49 — 1055.15 64.59

STATION 244+00.00

1055.29

STA. 243+50

END AREA		VOLUME	
CUT	FILL	CUT	FILL
8	298		
		14	574
7	321		
		12	636
5	365		
		31	701
28	392		
		54	706
30	370		
		68	612
43	290		

CALCULATED J.L.G.
CHECKED W.G.Y.

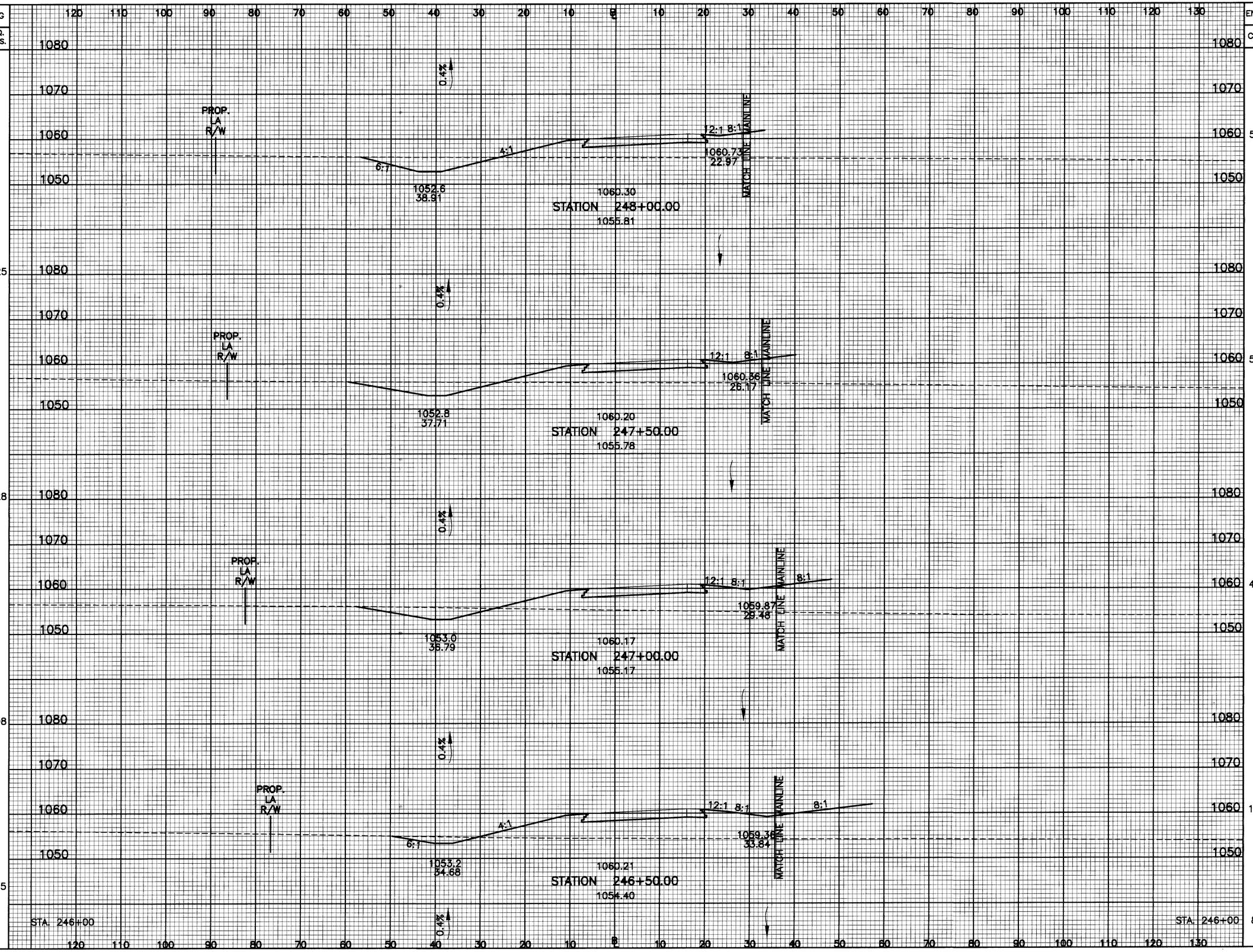
RAMP "D" X-SECTIONS STATION 244+00 TO STATION 246+00

FAY-35-2.57

220
522

Q:\3590\90RDX4

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
93	525	57	168	105	326	
96	528	56	184	95	388	
94	508	46	234	59	475	
89	475	17	278	24	534	
82	STA. 246+00	8	298			

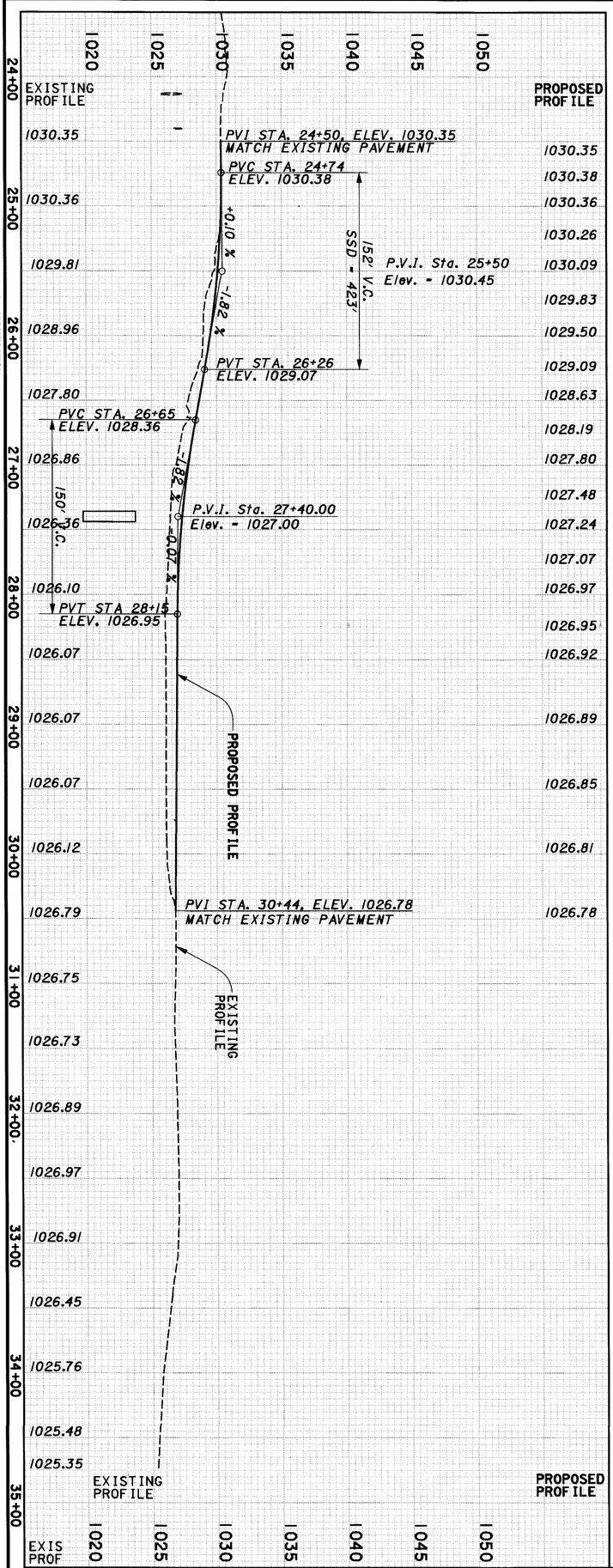


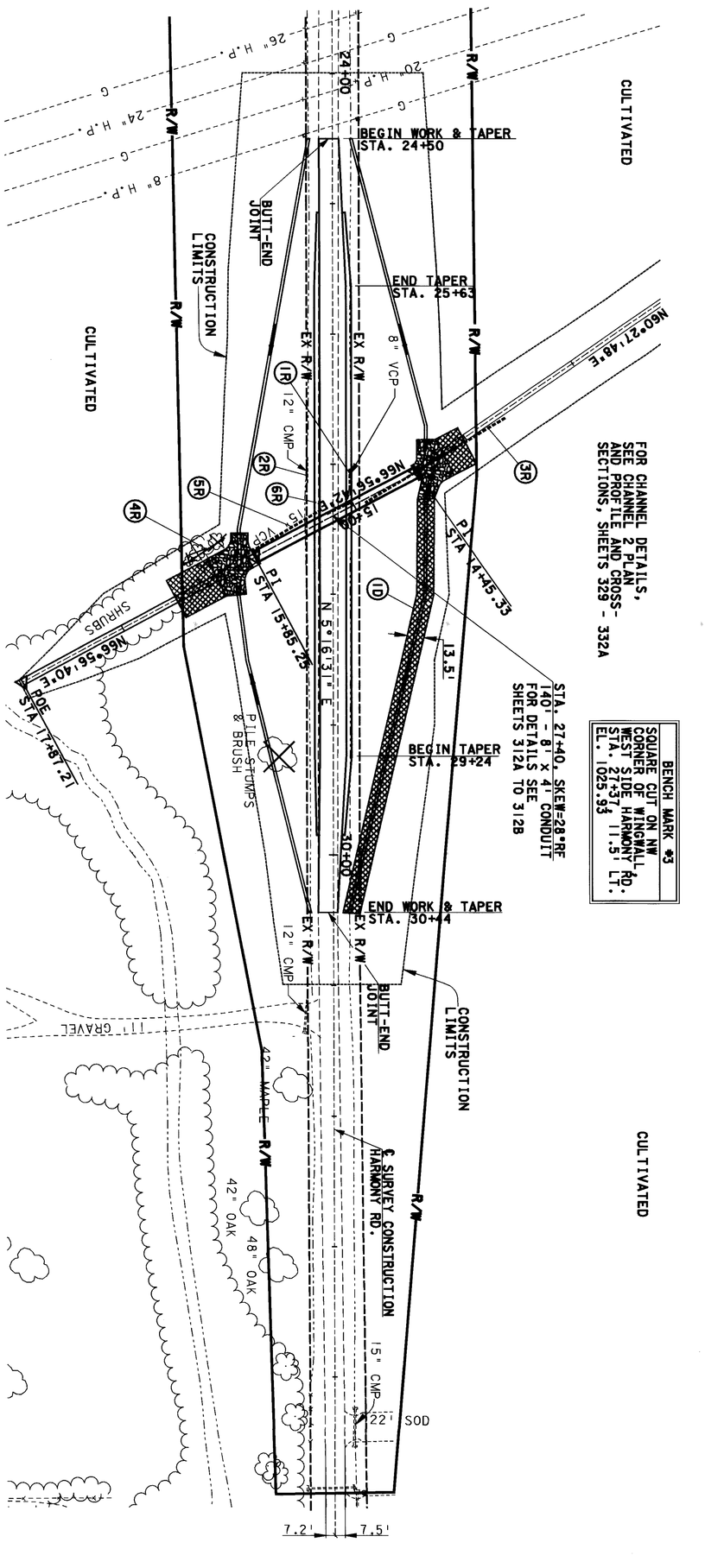
RAMP "D" X-SECTIONS STATION 246+50 TO STATION 248+00

FAY-35-2.57

221
522

Q:\3590\90RDX5





REF NO.	STATION FROM	STATION TO	SIDE	202 FENCE REMOVED	202 PIPE REMOVED UNDER 24"	670 DITCH EROSION PROTECTION
				L.F.	L.F.	SQ.YD.
1R	31+09	31+39	RT.		30	
2R	34+24	34+64	LT.		30	
3R	26+65	27+30	LT		135	
4R	27+56	27+80	RT.	40		
5R	27+38	28+00	RT.		125	
6R	27+30	27+38	L/R		26	
1D	27+20	34+00	LT.			464
TOTALS CARRIED TO SHEET 33				40	346	464

221A 522	FAY-35-2.57	HARMONY ROAD PLAN AND PROFILE STA. 24+50 TO STA. 30+44	REVISED 11/99 BY FYC	CALCULATED JPC CHECKED KDO	0 50 25 100 HORIZONTAL SCALE IN FEET

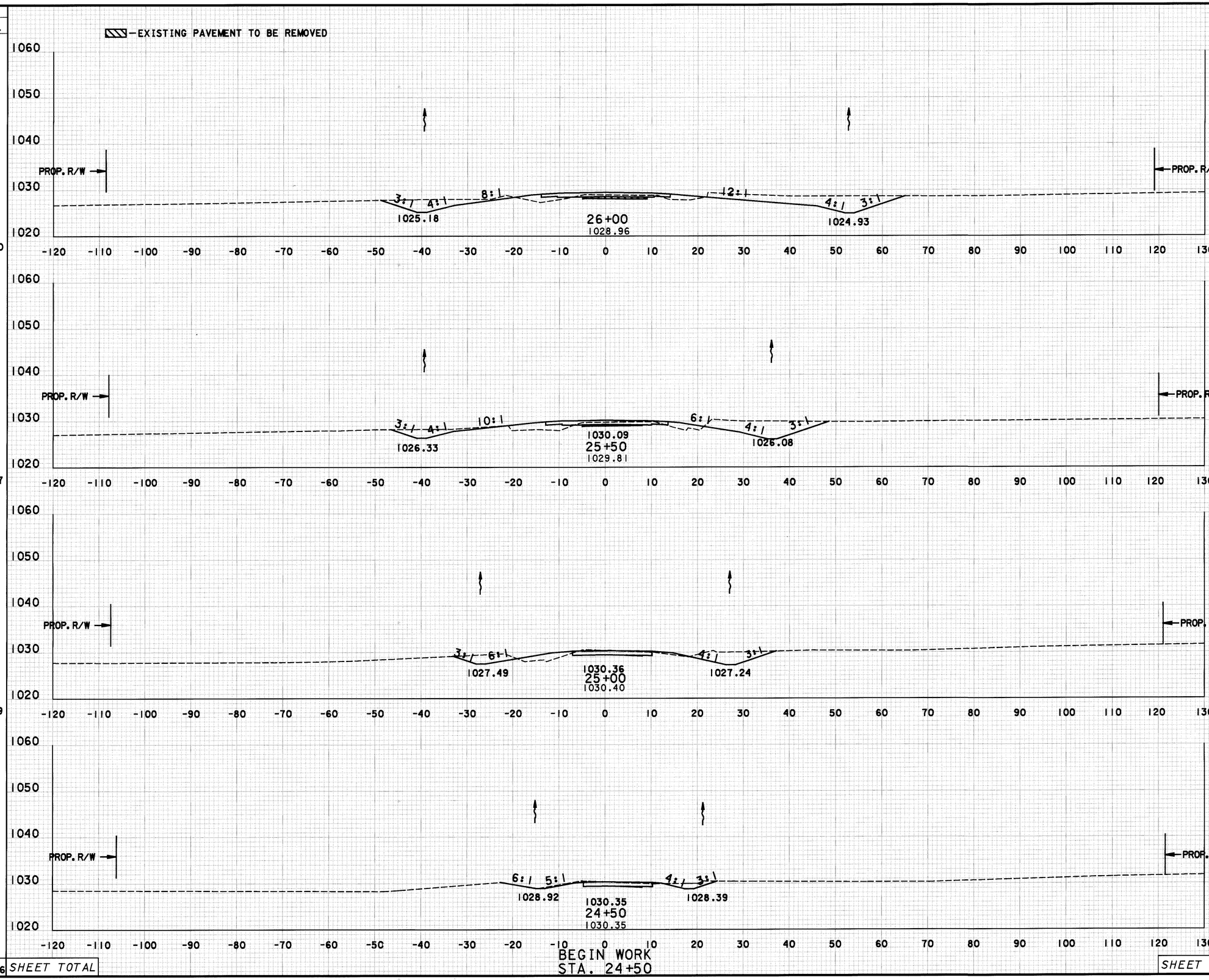
CROSS SECTIONS
HARMONY ROAD STA. 24+50 TO STA. 26+00

FAY-35-2.57

22IB
522

CALCULATED	CHECKED
JPC	KDO

REVISED	BY
11/99	FYC

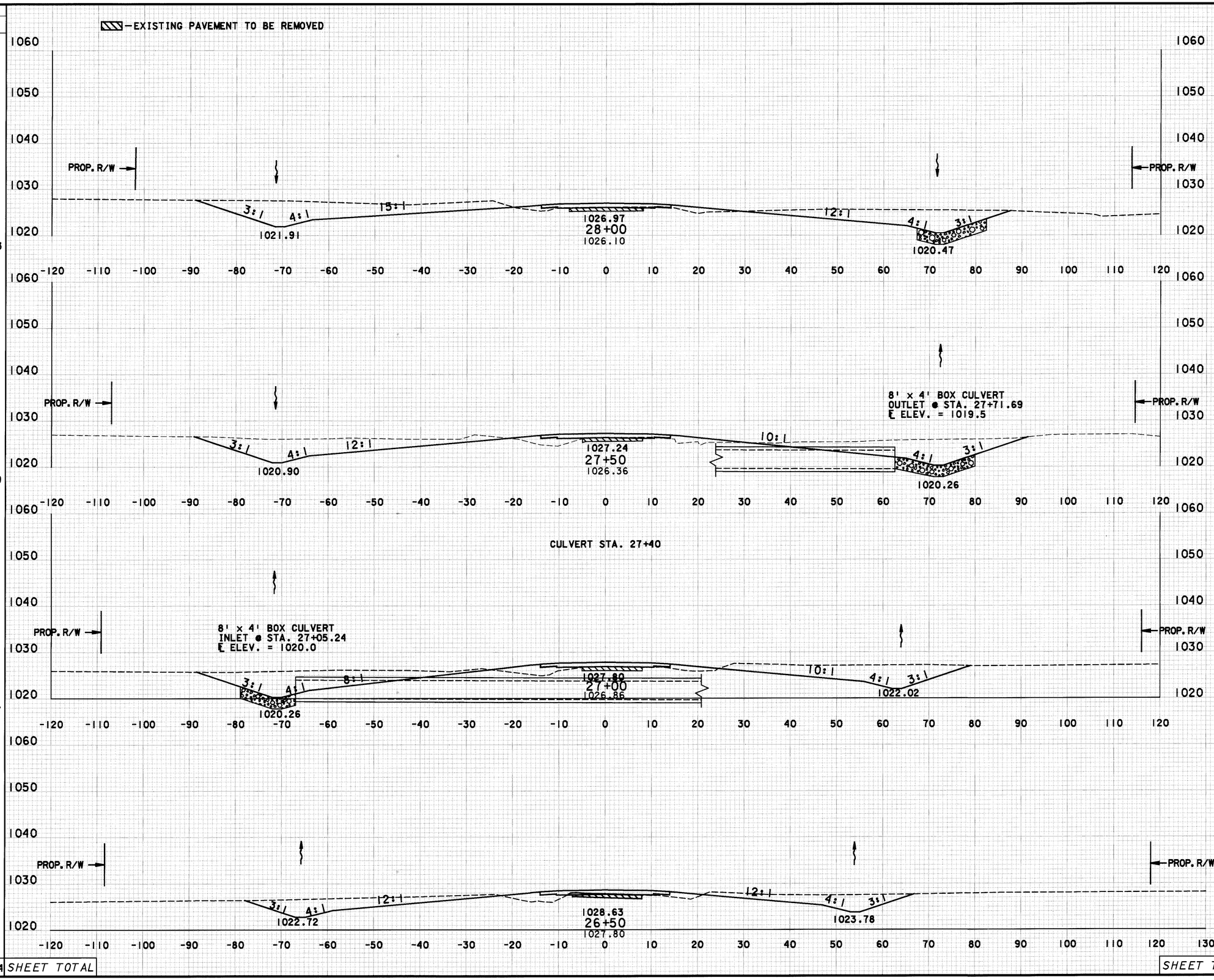


SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
90	450	132.0	16.0	212	37
72	347	97.0	24.0	150	38
53	239	65.4	17.0	83	16
33		24.8	0.3		
	1036 SHEET TOTAL			445	91

BEGIN WORK
STA. 24+50

I:\pr\project\fay35\fay-35-2.57\gxharmny.dgn 03-DEC-1999 9:17AM fcheek

SEEDING END WIDTH	SEEDING SQ. YDS.
150	833
150	789
134	697
117	575
	2894



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
301.9	17.6	496	48
233.3	34.5	387	66
185.1	36.7	306	58
145.3	25.9	256	39
SHEET TOTAL		1445	211

CALCULATED JPC
CHECKED KDO
REVISED 11/99
BY FYC

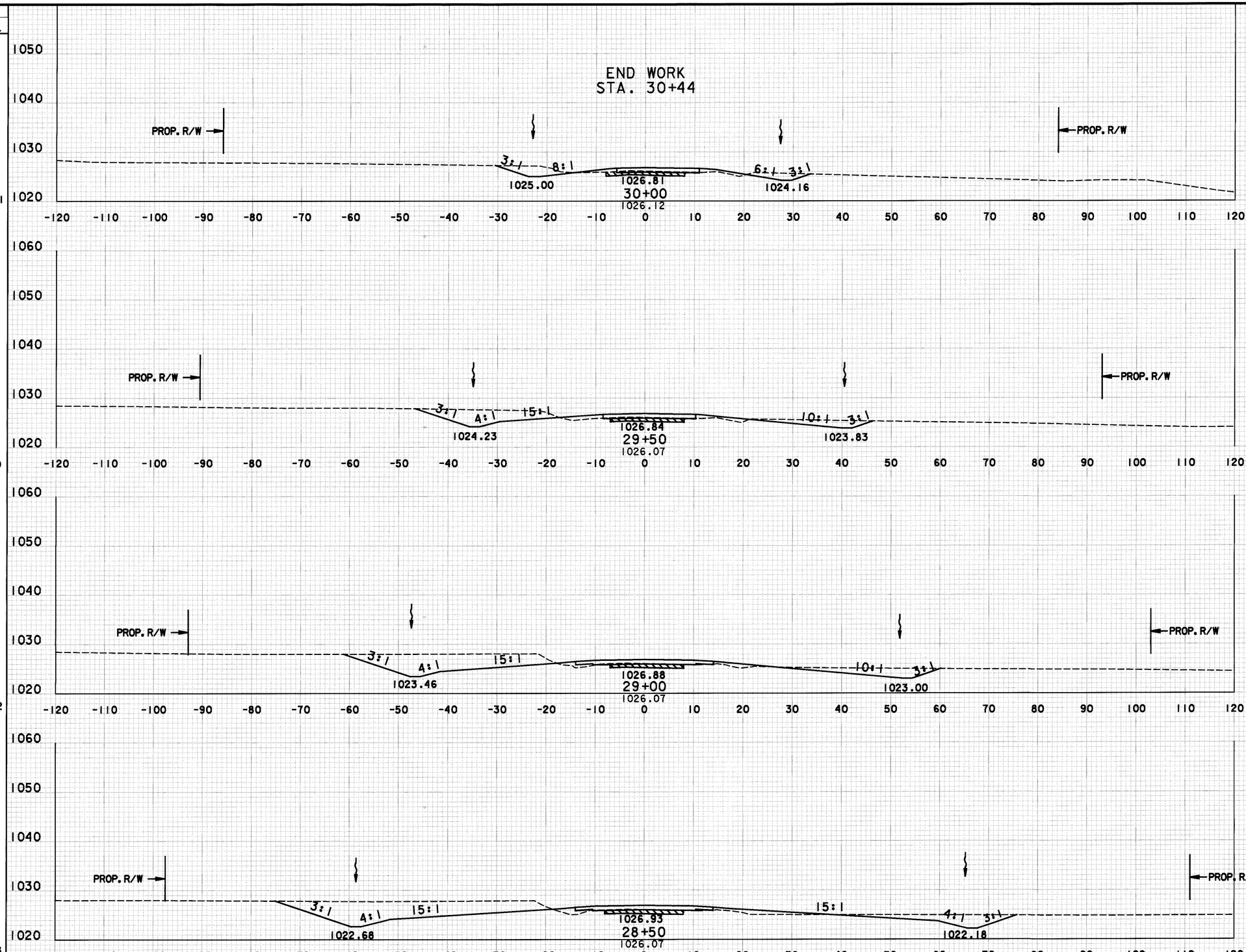
CROSS SECTIONS
HARMONY ROAD STA. 26+50 TO STA. 28+00

FAY-35-2.57

22IC
522

I:\pr\project\fay35\fay-35-2.57\gx\harmny.dgn 03-DEC-1999 9:18AM fcheek

SEEDING END WIDTH	SEEDING SQ. YDS.
49	331
70	450
92	622
132	783
	2186 SHEET TOTAL



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
45.0	6.4	127	14
91.9	8.2	239	16
166.7	9.3	355	26
217.0	18.3	481	33
SHEET TOTAL		1202	89

CALCULATED JPC
CHECKED KDO
REVISED 11/99
BY FYC

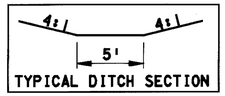
CROSS SECTIONS
HARMONY ROAD STA. 28+50 TO STA. 30+00

FAY-35-2.57

221D
522

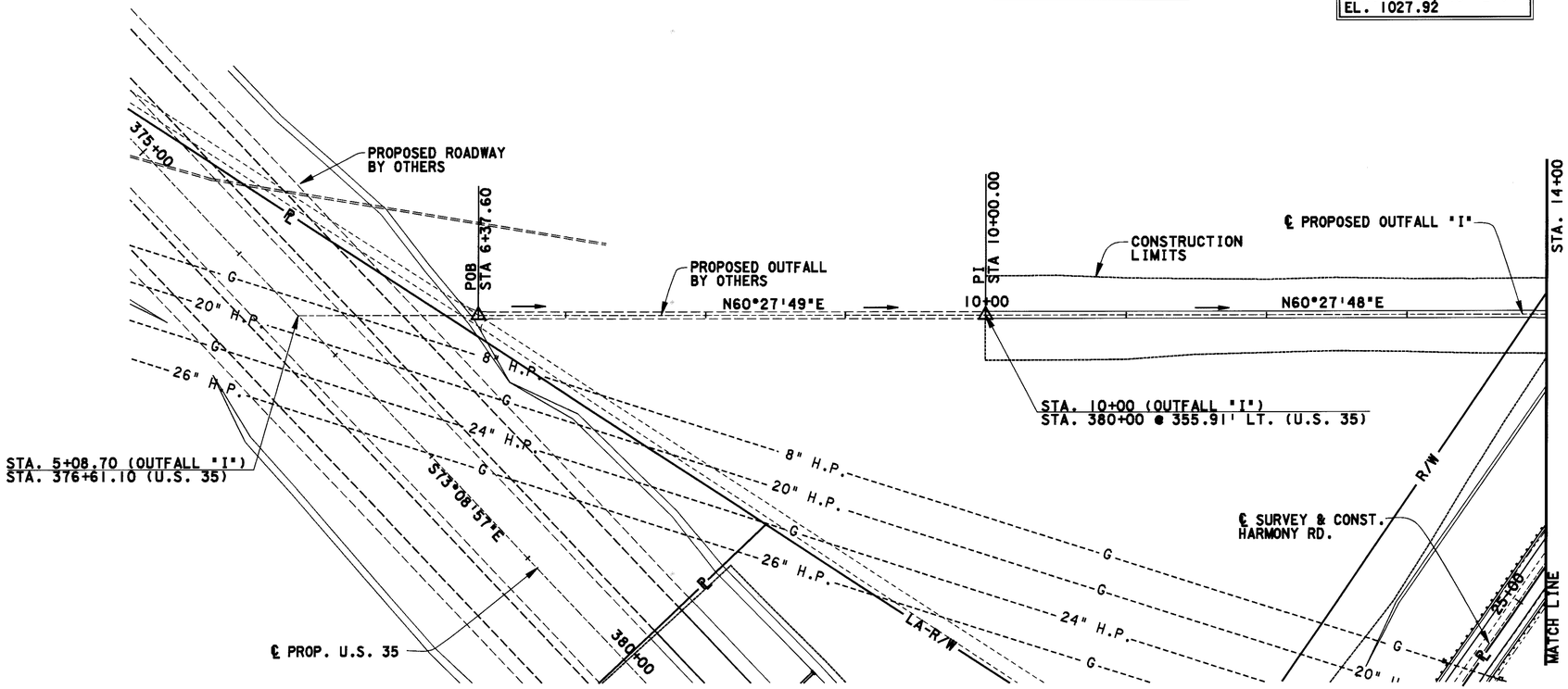
I:\pr\project\fay35\fay-35-2.57\gxharmny.dgn 03-DEC-1999 9:39AM fcheek

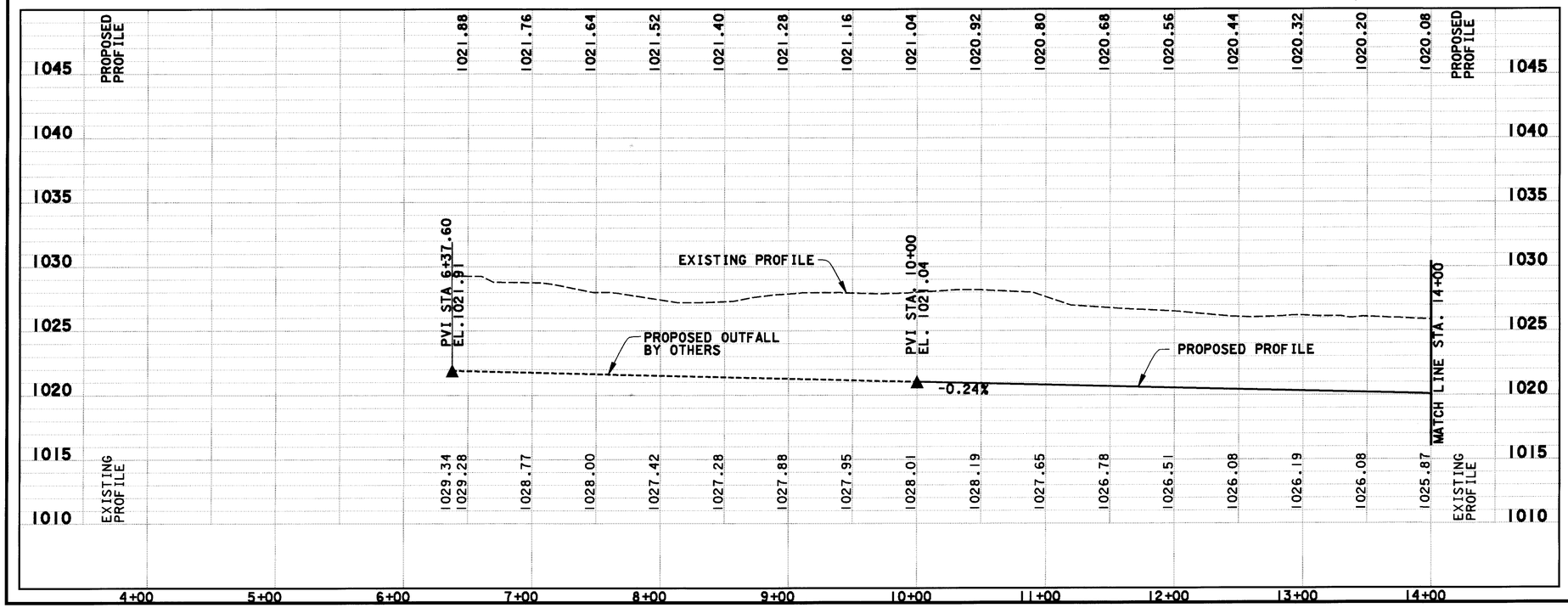
BENCH MARK #3
SQUARE CUT ON NW CORNER OF WINGWALL, WEST SIDE HARMONY RD. STA. 27+37, 11.5' LT. EL. 1025.93



TYPICAL DITCH SECTION

BENCH MARK #38
SQUARE CUT ON SW WINGWALL OF BRIDGE OVER CREEK ON HARMONY RD. NORTH OF PROPOSED US 35 STA. 37+47, 15' LT. EL. 1027.92





REF NO.	STATION FROM	STATION TO	SIDE

TOTALS CARRIED TO GENERAL SUMMARY

CALCULATED PJL
CHECKED JPC

PLAN AND PROFILE OUTFALL "I" STA. 6+37.60 TO STA. 14+00

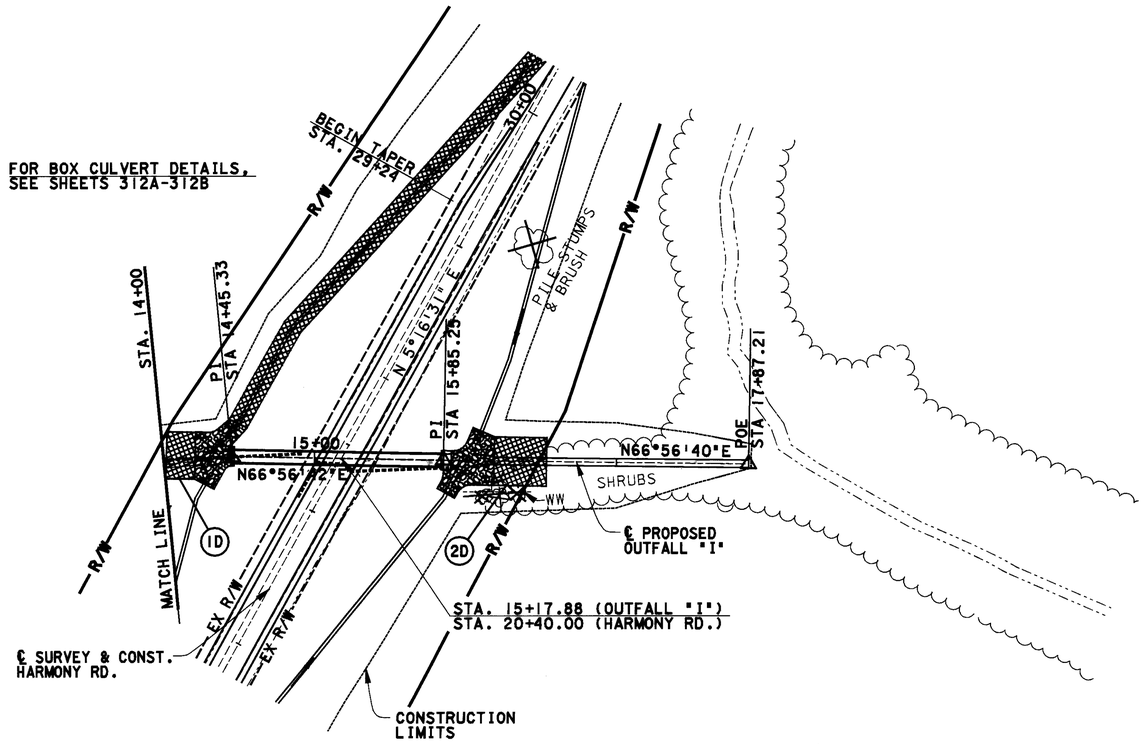
FAY-35-2.57

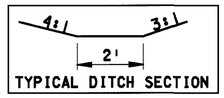
22 IE
522

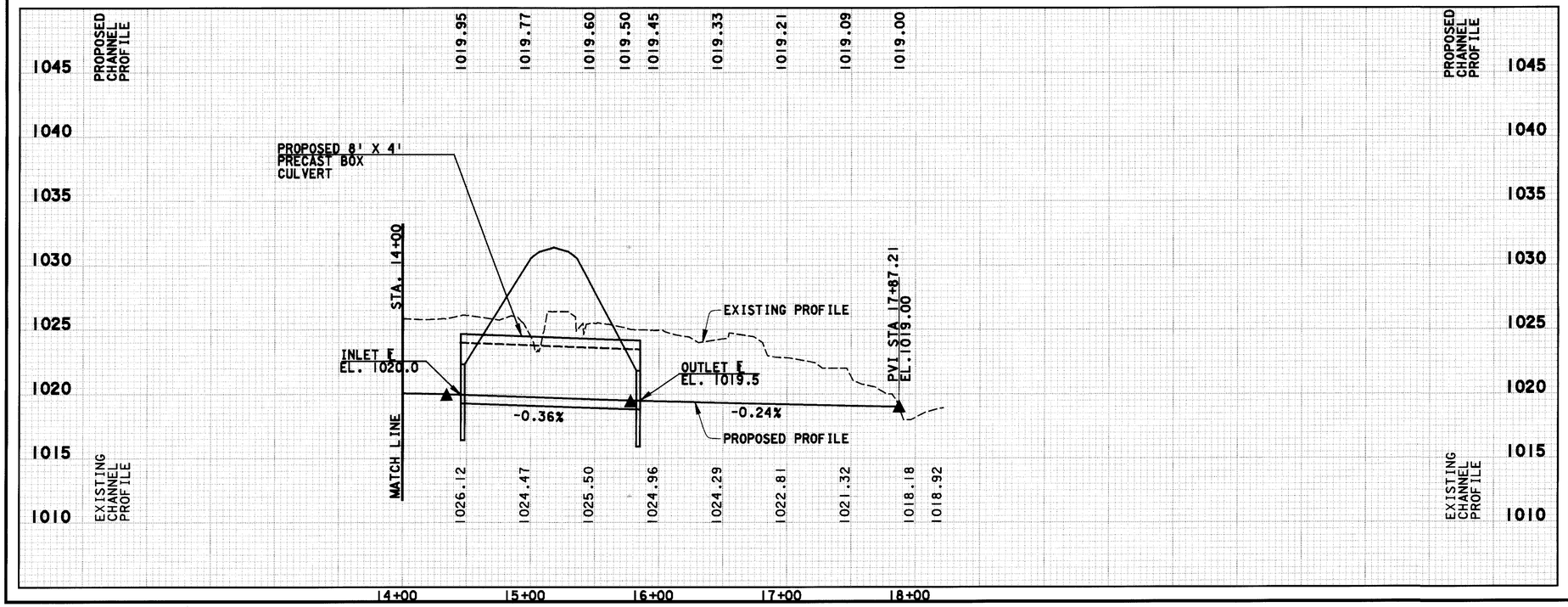
I:\pr\project\fay35\fay-35-2.57\ 08912dfq.dgn

BENCH MARK #3
SQUARE CUT ON NW CORNER OF WINGWALL, WEST SIDE HARMONY RD.
STA. 27+37, 11.5' LT.
EL. 1025.93

BENCH MARK #38
SQUARE CUT ON SW WINGWALL OF BRIDGE OVER CREEK ON HARMONY RD. NORTH OF PROPOSED US 35
STA. 37+47, 15' LT.
EL. 1027.92







REF NO.	STATION FROM	STATION TO	SIDE	670 DITCH EROSION PROTECTION SQ. YD.
1D	14+00	14+20		66
2D	16+18	16+51		124
TOTALS CARRIED TO GENERAL SUMMARY				190

HORIZONTAL SCALE IN FEET
0 25 50 100

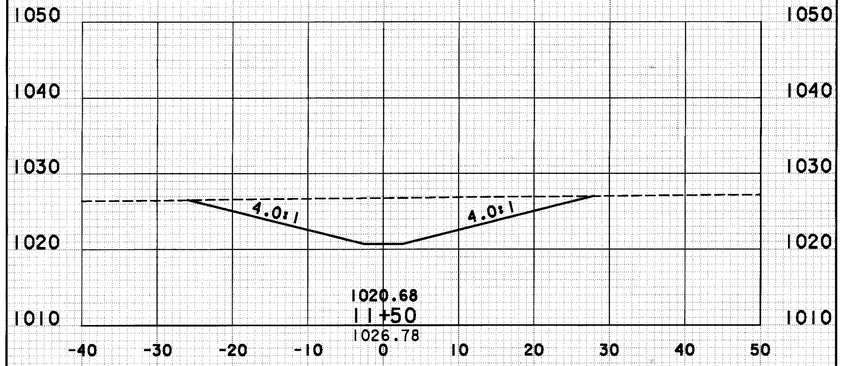
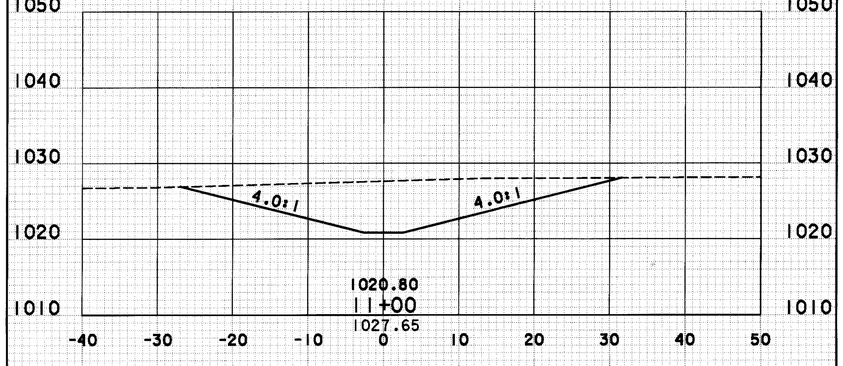
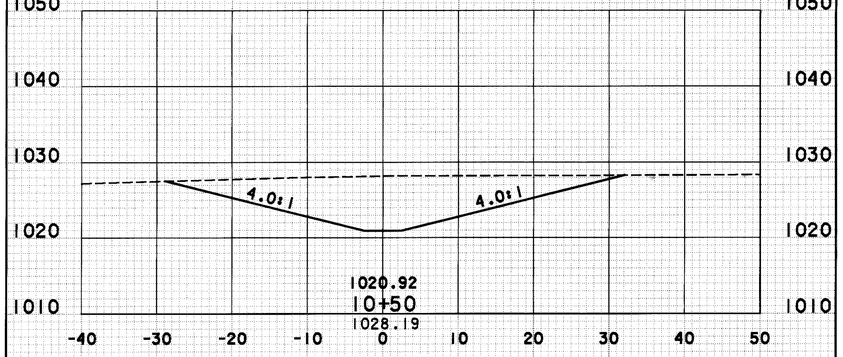
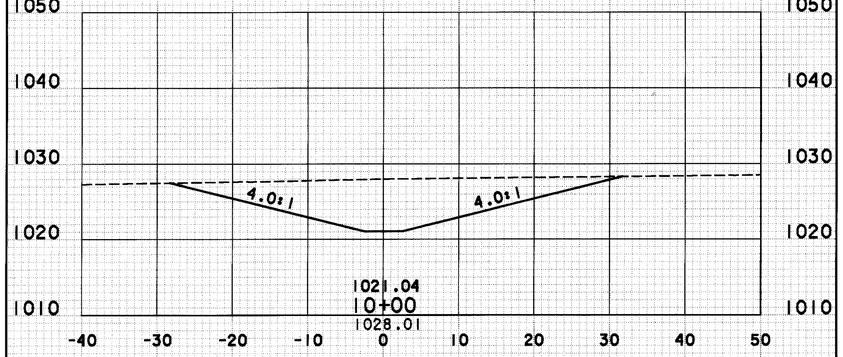
REVISED	CALCULATED	CHECKED
11/99	PJL	JPC
BY		
FYC		

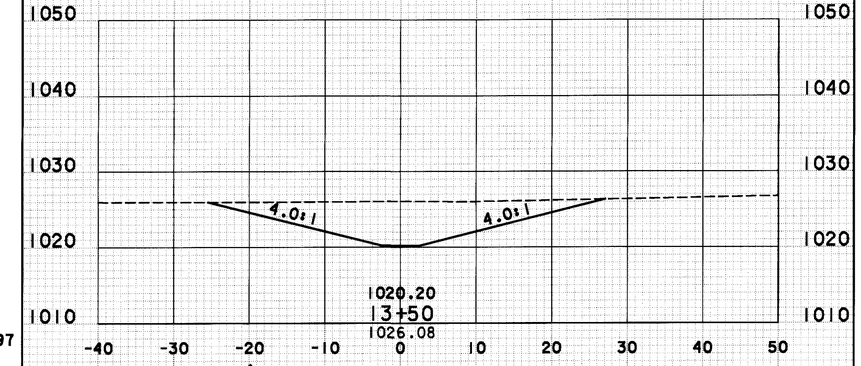
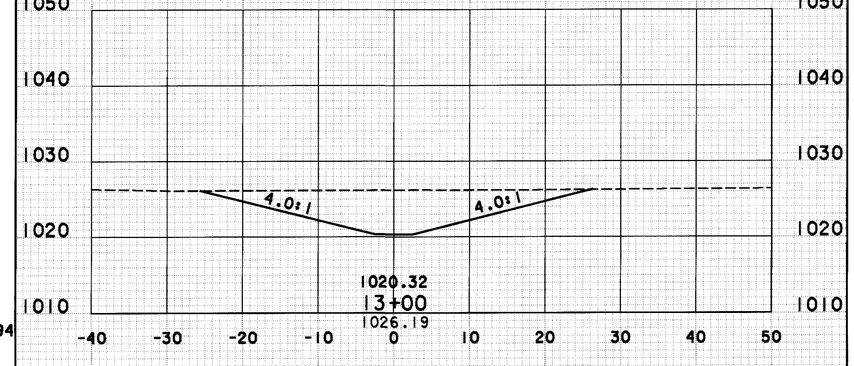
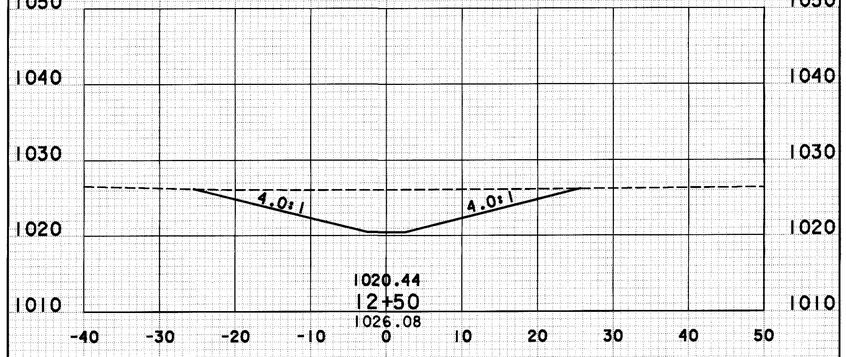
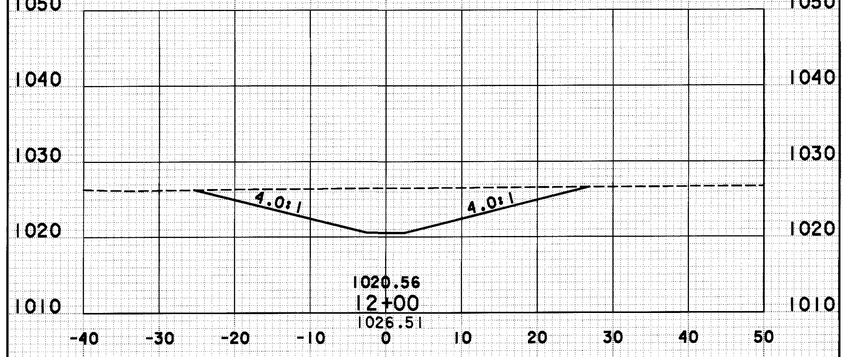
PLAN AND PROFILE OUTFALL "I"
STA. 14+00 TO STA. 18+21.45

FAY-35-2.57

22 IF
522

i:\pr\project\fay35\fay-35-2.57\ 06912dfr.dgn

SEEDING			END AREA		VOLUME	
END WIDTH	SQ. YDS.		CUT	FILL	CUT	FILL
55	319		183	0	368	0
60	342		214	0	420	0
63	347		239	0	428	0
62			223	0		

SEEDING			END AREA		VOLUME	
END WIDTH	SQ. YDS.		CUT	FILL	CUT	FILL
54	297		168	0	309	0
53	294		166	0	298	0
53	297		156	0	302	0
54	303		170	0	327	

CALCULATED

CHECKED

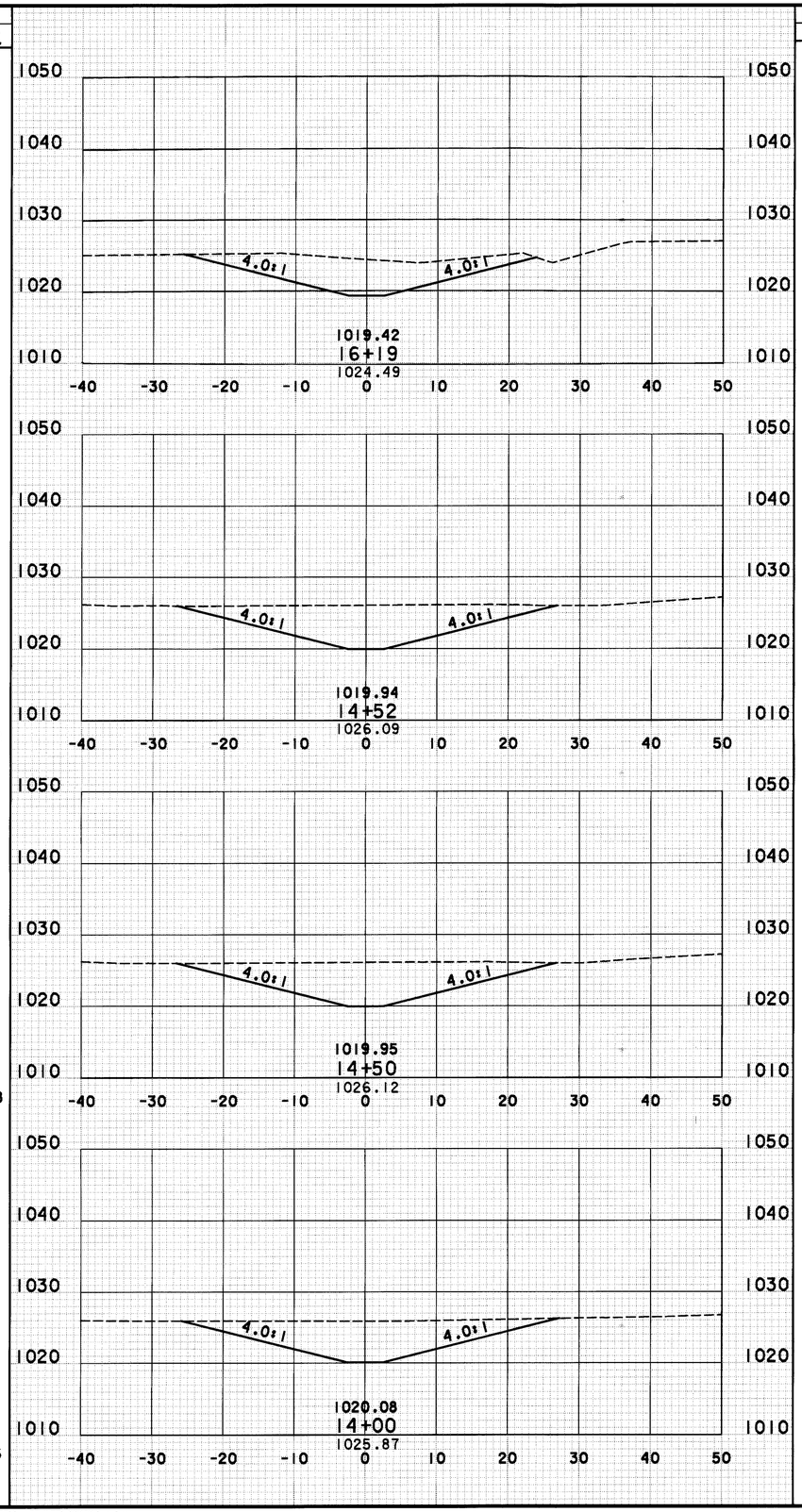
CROSS SECTION SHEET OUTFALL "I"
STA. 10+00 TO STA. 13+50

FAY-35-2.57

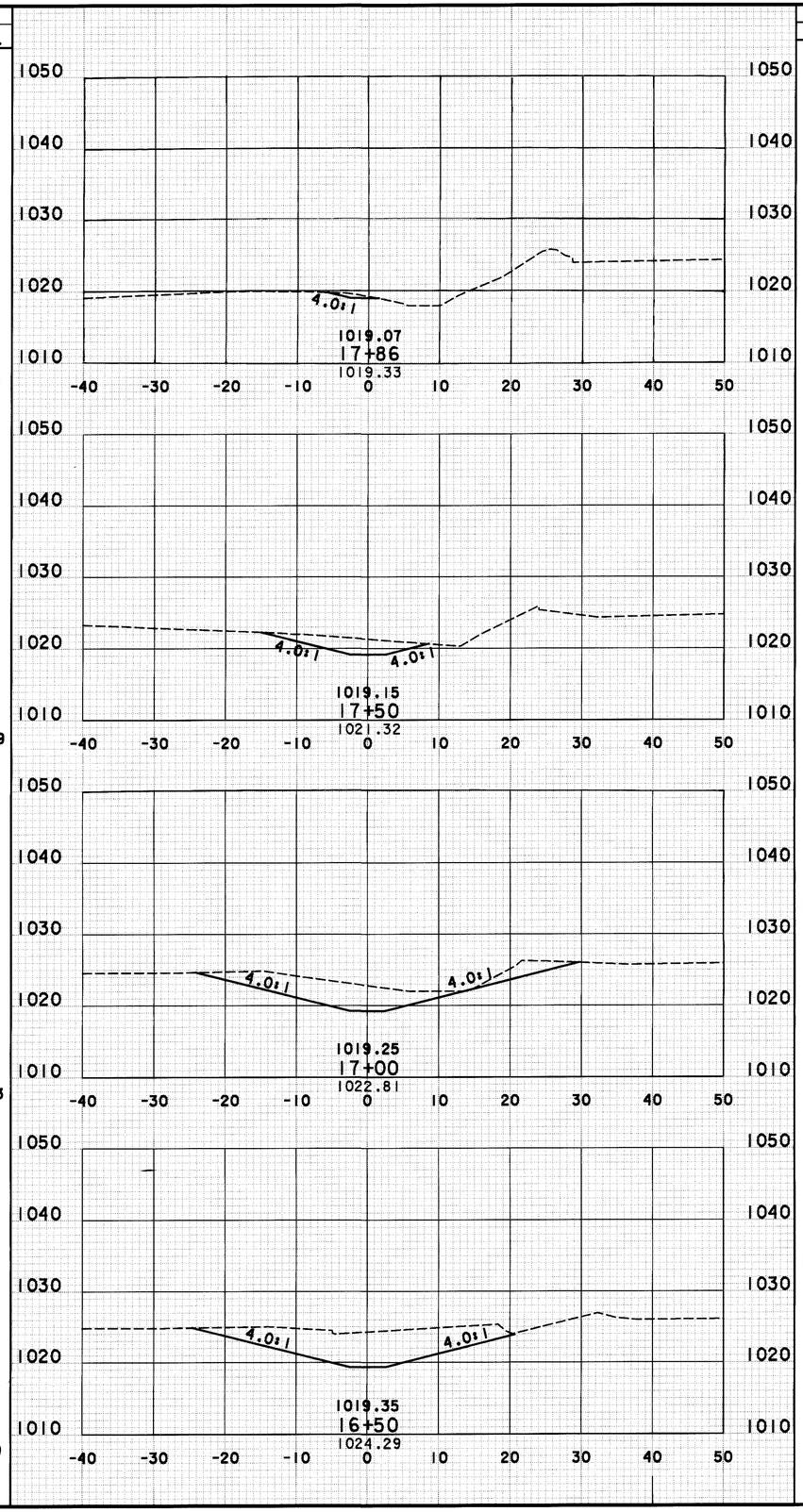
22IG
522

i:\pr\project\fay35\fay-35-2.57\ 06912dxn.dgn

SEEDING		END AREA		VOLUME	
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL
51		149	0		
55	12	181		14	0
55	308	182	0	331	0
56	306	175	0	318	0



SEEDING		END AREA		VOLUME	
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL
3		7	0		
54				27	0
24		34	0		
219				123	0
55		99	0		
283				222	0
47		141	0		
169				166	0



CALCULATED

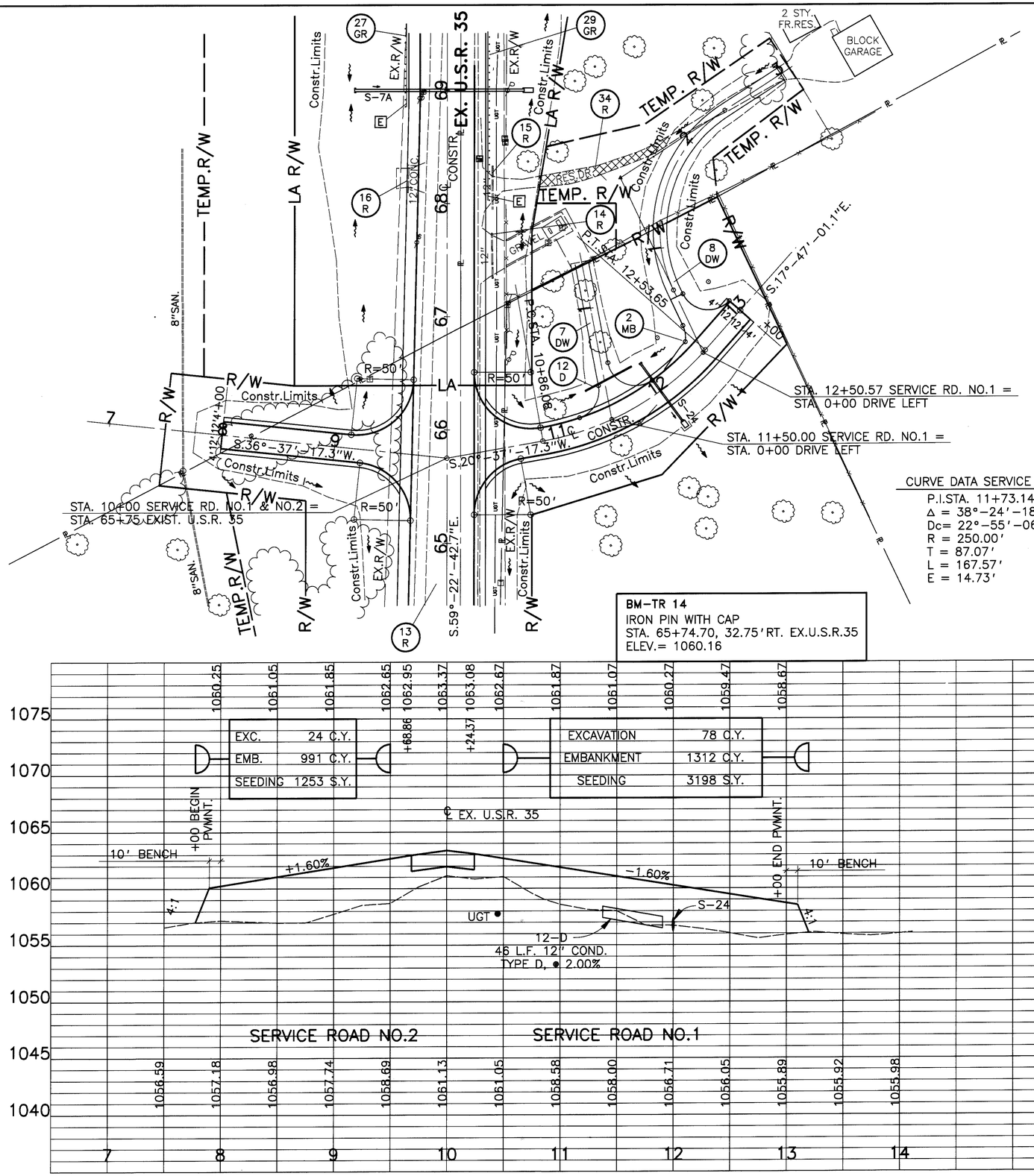
CHECKED

CROSS SECTION SHEET OUTFALL "I"
STA. 14+00 TO STA. 14+52

FAY-35-2.57

221H
522

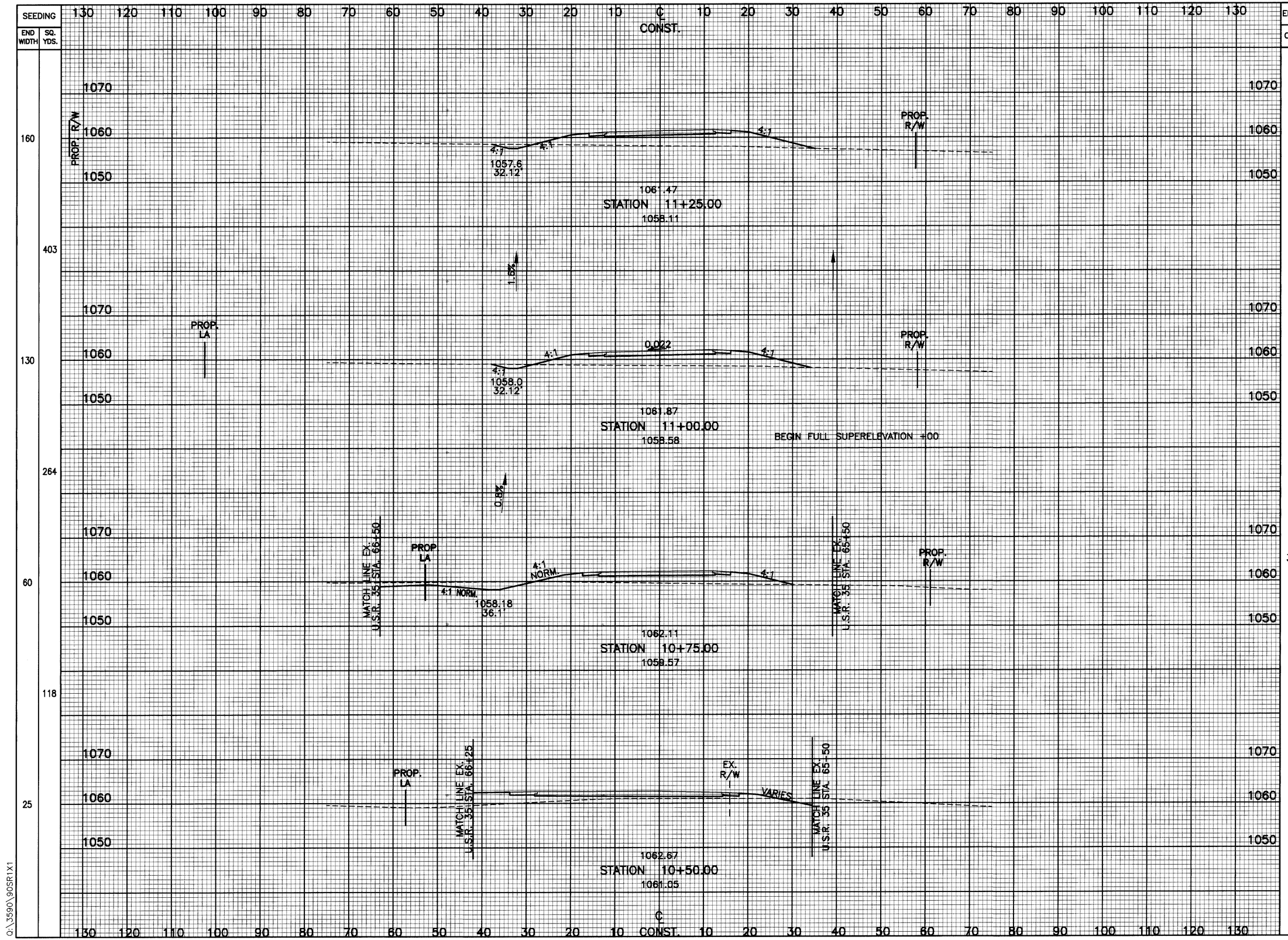
i:\pr\project\fay35\fay-35-2.57\ 06912dxn.dgn


TEMP. R/W
LA R/W
Constr.Limits
EX. U.S.R. 35
EX.R/W
TEMP. R/W
2 STY. FR.RES.
BLOCK GARAGE
TEMP. R/W
RES. DR.
TEMP. R/W
R/W
R/W

SEEDING END WIDTH	SEEDING SQ. YDS.
160	403
130	264
60	118
25	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
7	141	7	126
6	131	19	104
35	93	20	76
7	71		

CALCULATED J.L.G.
CHECKED W.G.Y.



SERVICE ROAD 1 X-SECTIONS STATION 10+50 TO STATION 11+25

FAY-35-2.57

223
522

Q:\3590\90SR1X1

SEEDING	
END WIDTH	SQ. YDS.
135	
	404
156	
	444
164	
	456
(93)	
164	
	450
160	

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

1070 / 1060 / 1050

PROP. R/W

END FULL SUPERELEVATION +30

4:1 6:1 4:1

1056.4
30.52'

1059.87

STATION 12+25.00

1056.42

HEADWALL HW=2.1M OR 2.2M
STA 12+00, 30.5' LT
℄ INLET = 1056.30

HEADWALL HW=2.1M OR 2.2M
STA 12+00, 38.5' RT
℄ OUTLET = 1056.04

4:1 4:1 4:1

1056.3
32.52

R.C.P. TYPE C

S-24

1060.27

STATION 12+00.00

1056.71

2.0%

1056.8
32.12'

1060.67

STATION 11+75.00

1056.74

12 D

STA. 11+90.63, 32.48' LT.
OUTLET 12" COND., TYPE D
℄ = 1056.48

2.0%

DEDUCT FOR DRIVE LT.

ADD FOR DRIVE LT

FOR DRIVE SEE SERVICE RD. No. 1 PLAN AND PROFILE (AMERITECH DRIVE)

4:1 4:1 4:1

1057.3
31.08'

12-D

1061.07

STATION 11+50.00

1058.00

12 D

STA. 11+37.67, 32.48' LT.
INLET 12" COND., TYPE D
℄ = 1057.40

STA. 11+25

STA. 11+25

℄ CONST.

130 120 110 100 90 80 70 60 50 40 30 20 10 10 20 30 40 50 60 70 80 90 100 110 120 130

END AREA		VOLUME		CALCULATED J.L.G.	CHECKED W.G.Y.
CUT	FILL	CUT	FILL		
3	140				
		6	129		
9	137				
		8	140		
8	165				
		7	137		
(0)	(59)				
5	130				
		6	126		
7	141				

SERVICE ROAD 1 X-SECTIONS STATION 11+50 TO STATION 12+25

FAY-35-2.57

224
522

Q:\3590\90SR1X2

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
129	226	0	0	0	33	
		0	87			
				0	110	
34	243	0	149	2	138	
141	383	3	149	3	134	
135		3	140			

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

1070 / 1060 / 1050

PROP. R/W

PROP. R/W

STATION 13+25.00
1056.12

END WORK +20

PROP. R/W

4:1 12:1 4:1

1058.67
STATION 13+00.00
1055.89

PROP. R/W

MATCH LINE SERVICE RD. NO. 1 DRIVEWAY STA. 0+50

4:1

1059.07
STATION 12+75.00
1055.58

PROP. R/W

PROP. R/W

6:1 NORM. 4:1

6:1
1056.5
51.0'

1059.47
STATION 12+50.00
1056.05

PROP. R/W

STA. 12+25 ℄ CONST. STA. 12+25

130 120 110 100 90 80 70 60 50 40 30 20 10 CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

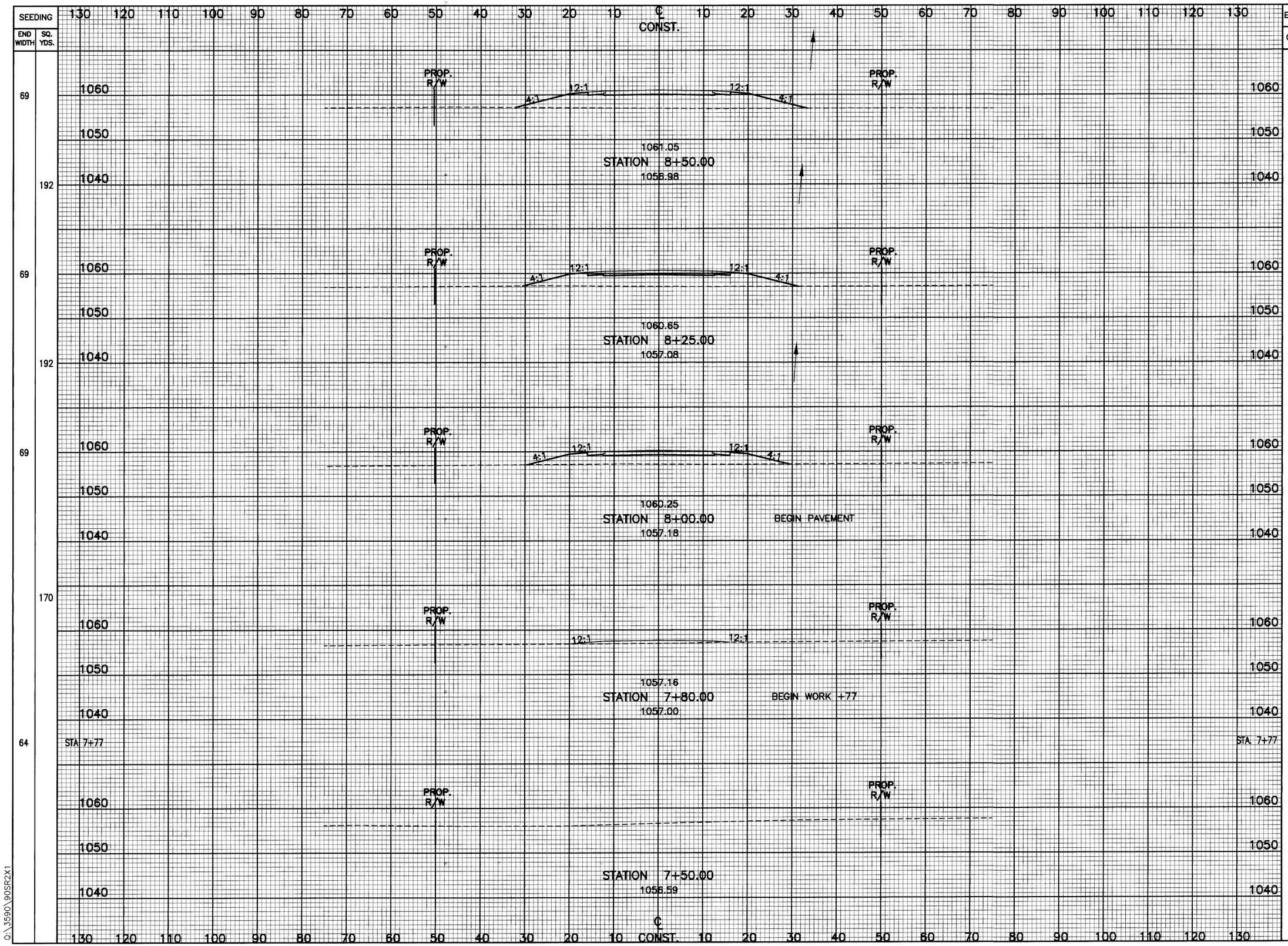
Q:\3590\90SR1X3

SERVICE ROAD 1 X-SECTIONS STATION 12+50 TO STATION 13+25

FAY-35-2.57

225
522

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
69	192	0	165	0	140	
69	192	0	136	0	113	
69	170	0	106	0	51	
		0	30	0	2	
64		0	0			



SERVICE ROAD 2 X-SECTIONS STATION 7+50 TO STATION 8+50

FAY-35-2.57

226
522

Q:\3590\90SR2X1

SEEDING	
END WIDTH	SQ. YDS.
17	118
68	190
69	196
72	195
69	

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

PROP. LA

MATCH LINE EX. U.S.R. 35 STA. 66+25

MATCH LINE EX. U.S.R. 35 STA. 65+50

1062.65
STATION 9+50.00
1058.69

4:1 NORM.

4:1

1056.5
39.92'

1062.25
STATION 9+25.00
1058.53

0.4%

MATCH LINE EX. U.S.R. 35 STA. 66+50

1056.6
37.92'

1061.85
STATION 9+00.00
1057.74

0.4%

PROP. R/W

1056.7
35.92'

1061.45
STATION 8+75.00
1058.99

1060 1050 1040

STA. 8+50

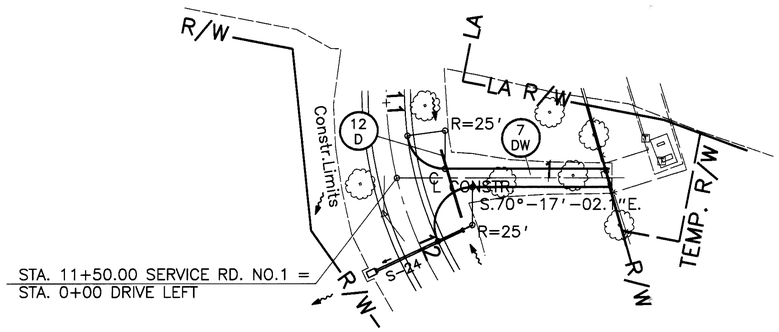
END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
CUT	FILL	CUT	FILL	
0	256			
		9	194	
19	163			
		12	154	
6	168			
		3	169	
0	196			
		0	168	
0	165			

SERVICE ROAD 2 X-SECTIONS STATION 8+75 TO STATION 9+50

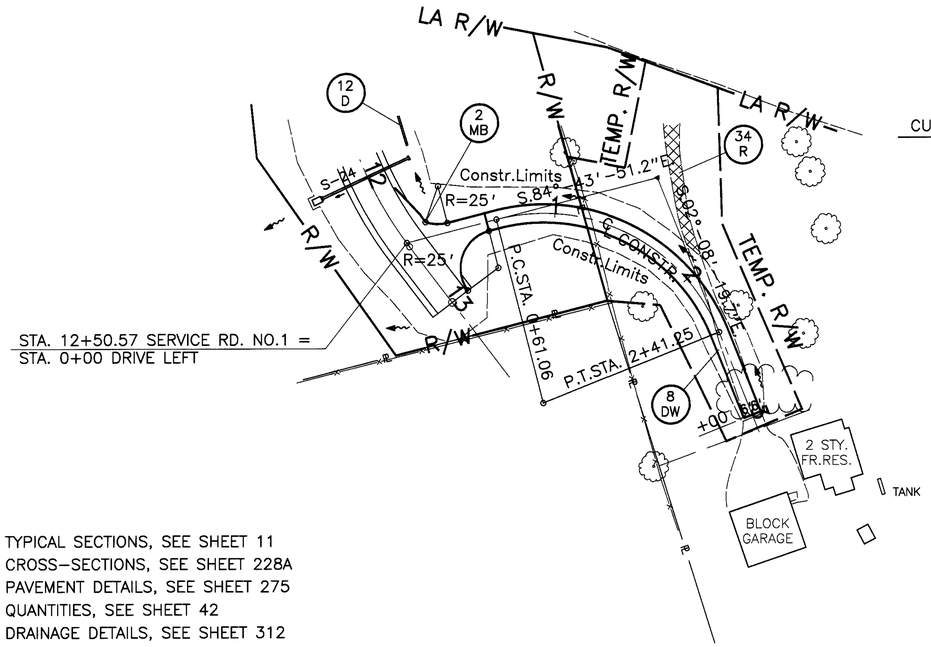
FAY-35-2.57

227
522

Q:\3590\90SR2X2



STA. 11+50.00 SERVICE RD. NO.1 =
STA. 0+00 DRIVE LEFT

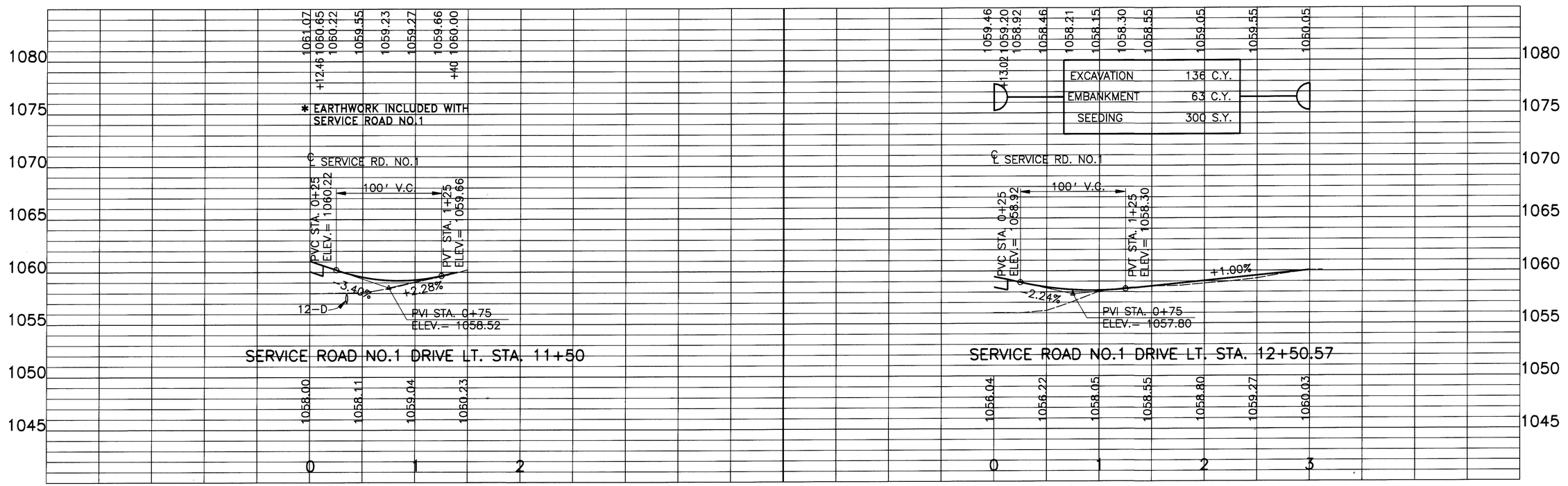


STA. 12+50.57 SERVICE RD. NO.1 =
STA. 0+00 DRIVE LEFT

CURVE DATA SERVICE RD. NO.1 DRIVE

P.I.STA. 1+70.86
Δ = 82°-35'-31.5" RT.
Dc= 45°-50'-12"
R = 125.00'
T = 109.80'
L = 180.19'
E = 41.37'

FOR TYPICAL SECTIONS, SEE SHEET 11
FOR CROSS-SECTIONS, SEE SHEET 228A
FOR PAVEMENT DETAILS, SEE SHEET 275
FOR QUANTITIES, SEE SHEET 42
FOR DRAINAGE DETAILS, SEE SHEET 312



SERVICE ROAD NO.1 DRIVE LT. STA. 11+50

SERVICE ROAD NO.1 DRIVE LT. STA. 12+50.57

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED | CHECKED

PLAN & PROFILE
SERVICE ROAD NO.1 DRIVES LEFT

FAY-35-2.57

228
522

SEEDING END WIDTH	SEEDING SQ. YDS.
4	61
18	97
17	100
19	42
11	

INCLUDED WITH SERVICE ROAD 1

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

1060.05
STATION 3+00.00
1060.03

1059.55
STATION 2+50.00
1058.27

1059.05
STATION 2+00.00
1058.80

1058.55
STATION 1+50.00
1058.55

1058.15
STATION 1+00.00
1058.05

1058.46
STATION 0+50.00
1058.22

PROP. TEMP. R/W
PROP. TEMP. R/W
EX. R/W
PROP. R/W
PROP. R/W
EX. P/L
L/A R/W
PROP. R/W
PROP. R/W
PROP. R/W

12:1 12:1 6:1
12:1 12:1 6:1 6:1
6:1 6:1 4:1

1+25 AHD. ONLY

END PAVEMENT +54.47
BEGIN GRAVEL

1060
1050

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
10	10		
15	3	24	13
14	3	27	6
22	0	34	3
15	0	35	0
3	44	16	41

CALCULATED J.L.G.
CHECKED W.G.Y.

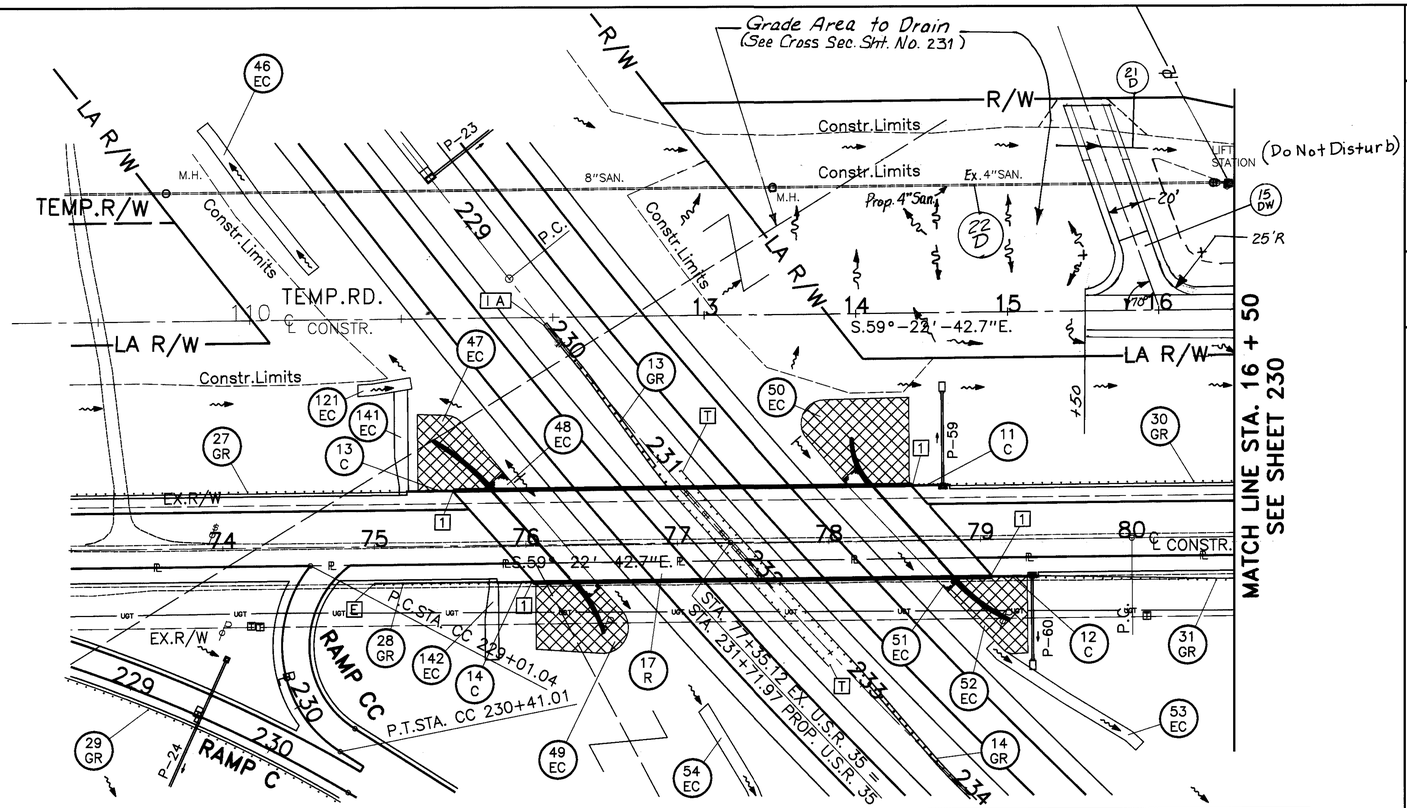
SERVICE ROAD 1-DRIVE LT. X-SECTIONS STATION 0+50 TO STATION 3+00

FAY-35-2.57

228A
522

Q:\3590\90sr1dr1

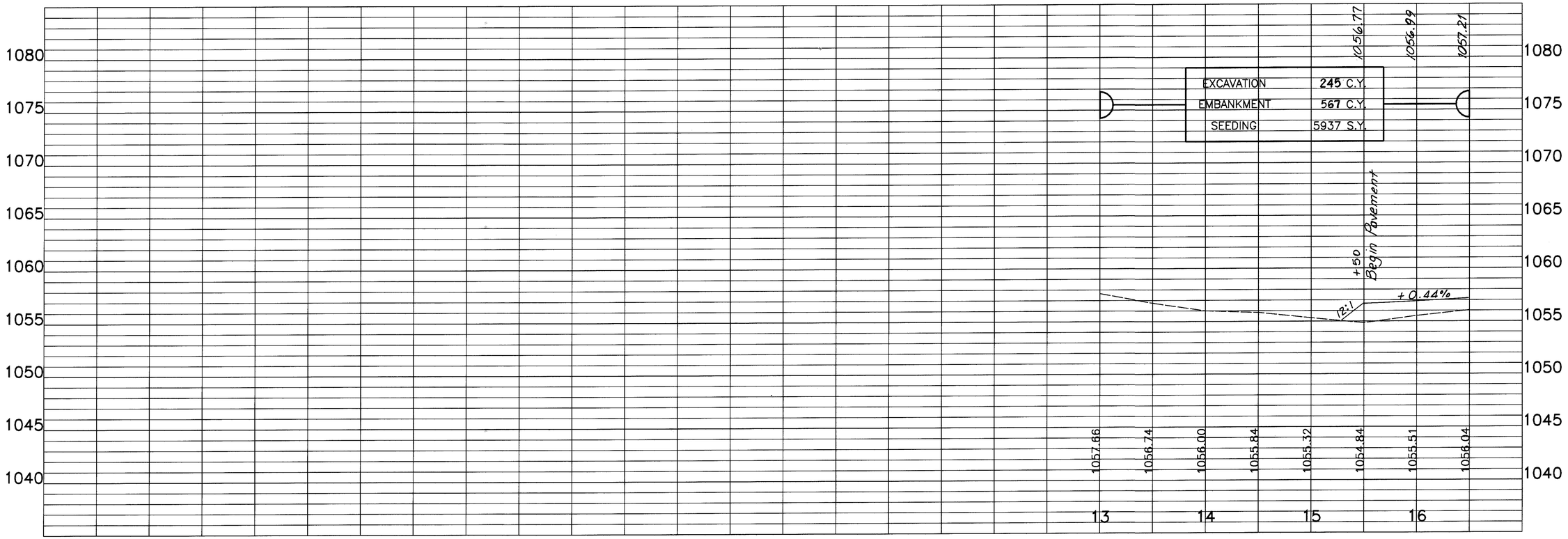
BM92–TR 2
IRON PIN WITH CAP
STA. 76+57.87, 31.40' RT. EX.U.S.R.35
ELEV.= 1061.23



FOR CROSS–SECTIONS, SEE SHEETS 231–232
FOR QUANTITIES, SEE SHEET 42 AND 39

FOR EXIST. USR 35, SEE SHEET 184
FOR PROP. USR 35, SEE SHEET 61

[1] BRIDGE TERMINAL ASSEMBLY, TYPE 1
[I A] IMPACT ATTENUATOR, TYPE 1–98 (BIDIRECTIONAL)
[T] ANCHOR ASSEMBLY, TYPE T
[E] ANCHOR ASSEMBLY, TYPE E–98



EXCAVATION	245 C.Y.
EMBANKMENT	567 C.Y.
SEEDING	5937 S.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED | CHECKED

PLAN & PROFILE SERVICE ROAD NO.3
STA. 13 + 50 TO STA. 16 + 50

FAY–35–2.57

229
522

BM-TR 22
IRON PIN WITH CAP
STA. 89+19.33, 23.52' RT. EX.U.S.R.35
ELEV.= 1059.02

CURVE DATA SERVICE RD. NO.3
P.I.STA. 28+80.92
Δ = 90°-00'-00" RT.
Dc= 57°-17'-45"
R = 100.00'
T = 100.00'
L = 157.08'
E = 41.42'

SEE SHEET 229
MATCH LINE STA. 16+50

EXACT LOCATION UNKNOWN
CONTRACTOR TO LOCATE

R/W
8"SAN.
Constr.Limits
S-21
S-22
S-8
S-9
S-13
LA
EX.R/W
S.59°-22'-42.7"E.
6" TILE(ABANDON)
12"CONC.
12"CMP
EX. U.S.R. 35
TEMP.R/W
TEMP. RD. & CONSTR.
R/W
LA R/W
Constr.Limits
TANK
WELL
WIND MILL
FLOWER BED W/ R.R.TIES
STY FR. RES.
CONC.
P.C.STA. 27+80.92
P.T.STA. 29+38.00
R=50'
S.30°-37'-17.3"W.
STA. 30+00 SERVICE RD. NO.3 =
STA. 93+00 EXIST. U.S.R. 35
ELEC. EASEMENT

[E] ANCHOR ASSEMBLY, TYPE E-98

FOR TYPICAL SECTIONS, SEE SHEET 11
FOR CROSS-SECTIONS, SEE SHEETS 232-236
FOR PAVEMENT DETAILS, SEE SHEET 274
FOR QUANTITIES, SEE SHEET 241,242
FOR DRAINAGE DETAILS, SEE SHEETS 309,310,311

FOR EXIST. USR 35, SEE SHEETS 184,185

EXCAVATION	1968 C.Y.
EMBANKMENT	5225 C.Y.
SEEDING	17.926 S.Y.

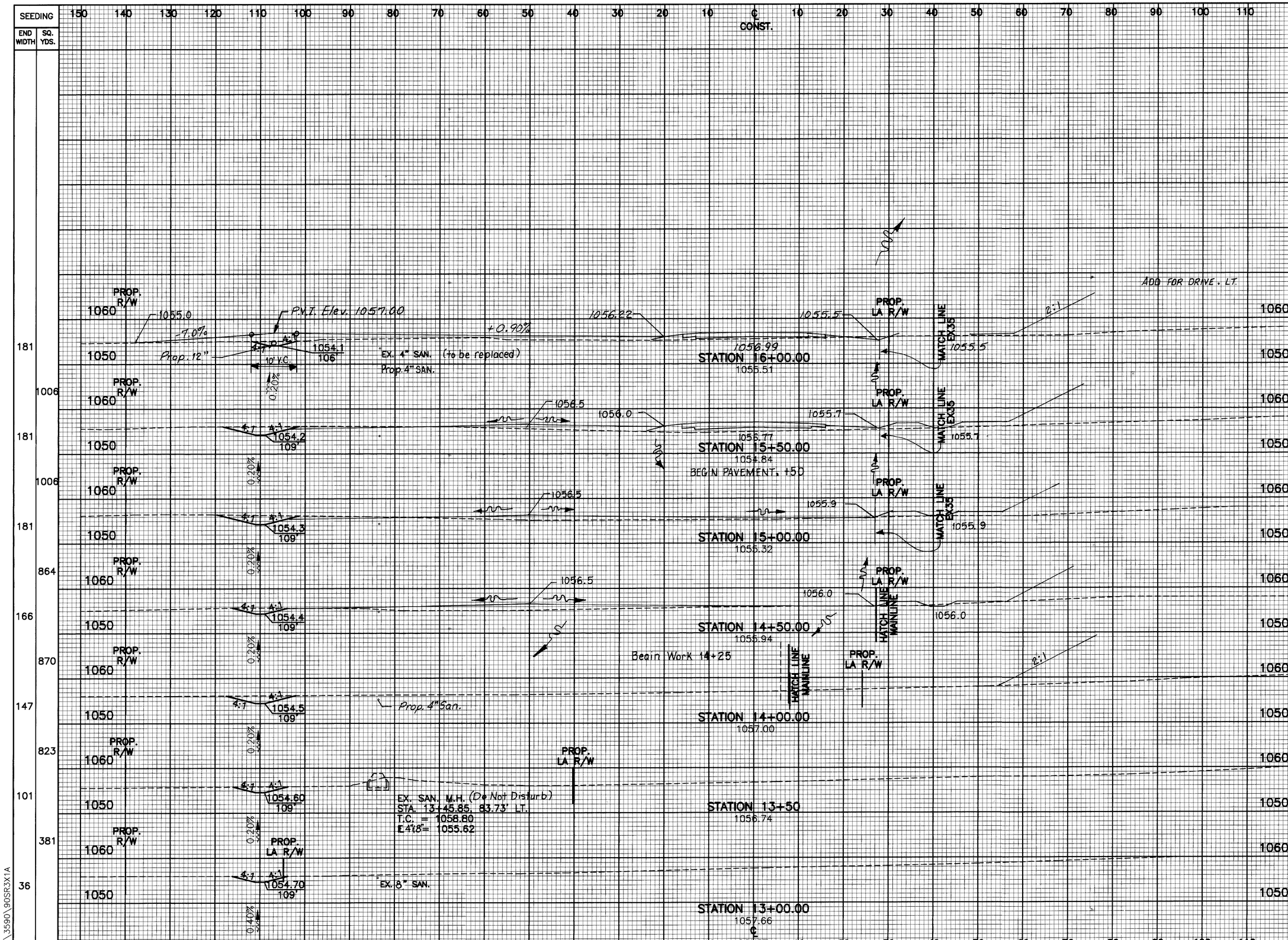
PVI STA. 21+50
ELEV.= 1059.41
+0.44%
-0.40%
+1.40%
PVC STA. 20+50
ELEV.= 1058.97
PVT STA. 22+50
ELEV.= 1059.01
PVC STA. 27+00
ELEV.= 1057.21
PVI STA. 29+00
ELEV.= 1058.21
PVI STA. 28+00
ELEV.= 1056.81
200' V.C.
EX.6"TILE
10-D
S-21
S-22
S-23
℄ EX. U.S.R. 35

HORIZONTAL SCALE IN FEET
0 25 50 100
CALCULATED
CHECKED

PLAN & PROFILE SERVICE ROAD NO.3
STA. 16+50 TO STA. 30+00

FAY-35-2.57

230
522



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
			127
7	19		
		34	56
28	92		
		57	86
33	80		
		40	166
10	99		
		25	92
16	0		
		26	0
12	0		
		21	0
10	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

SERVICE ROAD 3 X-SECTIONS STATION 13+00 TO STATION 16+00

FAY-35-2.57

Q:\3590\90SR3X1A

SEEDING	
END WIDTH	SQ. YDS.
114	1250
111	1345
131	373
137	432
174	987
181	

END AREA		VOLUME	
CUT	FILL	CUT	FILL
3	194	19	562
7	109	30	284
9	44	10	32
12	24	24	22
38	24	42	40
7	19		

CALCULATED J.L.G.
CHECKED W.G.Y.

HEADWALL HW2.2M
STA 19+34, 33' LT
℄ OUTLET = 1053.45

HEADWALL HW2.2M
STA 19+34, 30' RT
℄ INLET = 1053.73

RCP TYPE C

EX. 8" SAN.

PROP. R/W

PROP. LA R/W

MATCH LINE EX. U.S.R. 35

VARIES

0.20%

4:1

2:1

1053.5
36.16

1053.7
46.00

1053.9
80.0

1053.93
89.0

1054.0
96.5

1055.3

1058.31
STATION 19+00.00
1054.20

1057.87
STATION 18+00.00
1055.29

1057.43
STATION 17+00.00
1056.74

1057.32
STATION 16+75.00
1056.02

1055.1

1057.21
STATION 16+50.00
1056.04

EX. SAN. M.H. (LIFT STATION) (Do Not Disturb)
STA. 16+36.63, 83.32' LT.
TOP = 1088.2
℄ 4" = 1056.0
℄ 8" = 1044.0

STA.16+00

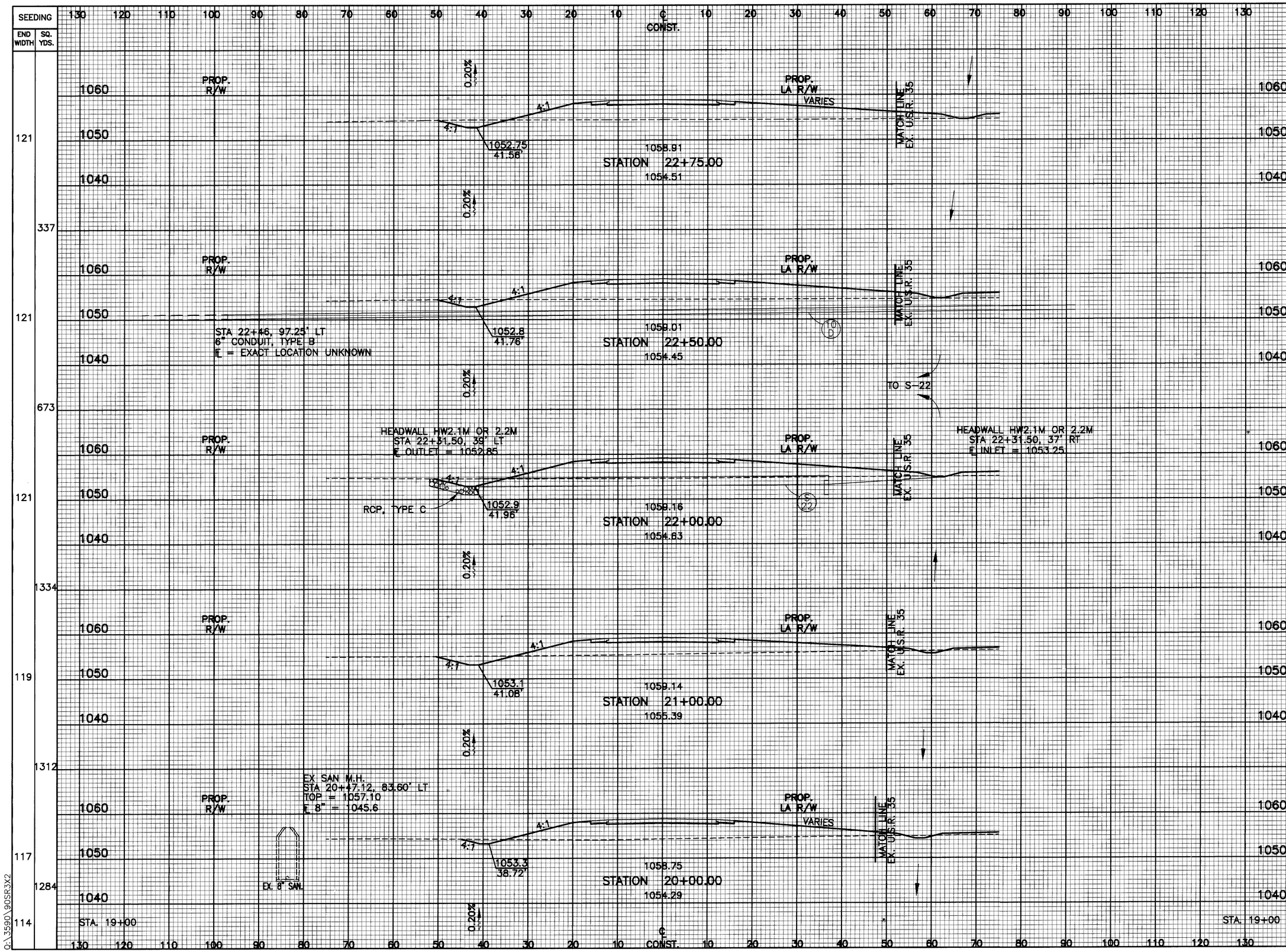
℄ CONST.

SERVICE ROAD 3 X-SECTIONS STATION 16+50 TO STATION 19+00

FAY-35-2.57

232
522

Q:\3590\90SR3X1



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
17	248		
		15	235
15	258		
		30	474
17	253		
		65	823
18	191		
		47	776
7	228		
		19	782
3	194		

CALCULATED J.L.G.
CHECKED W.G.Y.

SERVICE ROAD 3 X-SECTIONS STATION 20+00 TO STATION 22+75

FAY-35-2.57

Q:\3590\90SR3X2

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
122	678	129	2	188	24	
122	675	74	23	119	75	
121	673	54	58	87	160	
121	1345	39	114	112	652	
121	337	21	238	18	225	
121		17	248			

STATION 25+50.00
1057.81
1057.13
1052.2
39.36'
1056.05
24'
STATION 25+00.00
1058.01
1056.57
1052.3
39.76'
1056.25
24'
STATION 24+50.00
1058.21
1056.59
1052.4
40.16'
STATION 24+00.00
1058.41
1056.04
1052.5
40.56'
STATION 23+00.00
1058.81
1054.64
1052.7
41.36'

℄ TEMP. RD.
PROP. R/W
PROP. LA R/W
MATCH LINE EX. U.S.R. 35
TEMP. RD.
0.20%
0.40%
4:1

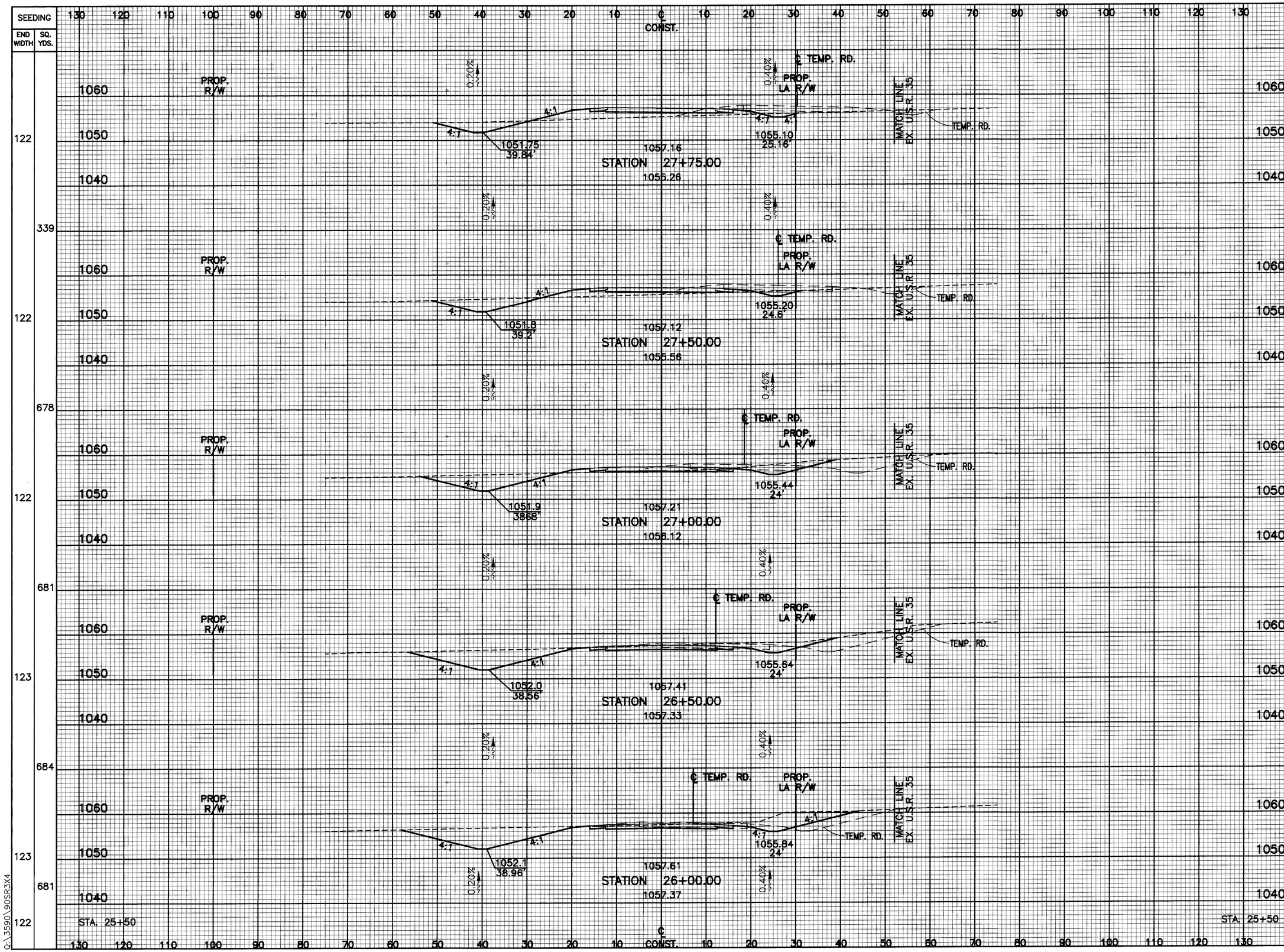
STA. 22+75
℄ CONST.
130 120 110 100 90 80 70 60 50 40 30 20 10 0 10 20 30 40 50 60 70 80 90 100 110 120 130

SERVICE ROAD 3 X-SECTIONS STATION 23+00 TO STATION 25+50

FAY-35-2.57

234
522

Q:\3590\90SR3X3



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
34	59		
		38	43
46	33		
		132	41
96	11		
		231	11
153	0		
		308	1
180	1		
		287	3
129	2		

CALCULATED J.L.G.
CHECKED W.G.Y.

SERVICE ROAD 3 X-SECTIONS STATION 26+00 TO STATION 27+75

FAY-35-2.57

Q:\3590\90SR3X4

| SEEDING | | |

BM–TR 105
IRON PIN WITH CAP
STA. 153+22.53, 14.55' LT.
ELEV.= 1048.47

BM–TR 1051
IRON PIN WITH CAP
STA. 160+24.53, 18.00' RT.
ELEV.= 1042.27

BEGIN WORK
STA. 152+75

MATCH LINE STA. 163 + 00
SEE SHEET 238

FOR TYPICAL SECTIONS, SEE SHEET 10
FOR CROSS–SECTIONS, SEE SHEETS 240–246
FOR QUANTITIES, SEE SHEETS 37,38,39,41,42

FOR DRIVE DETAILS, SEE SHEET 11

[E] ANCHOR ASSEMBLY, TYPE E–98
[T] ANCHOR ASSEMBLY, TYPE T

EXCAVATION	766 C.Y.
EMBANKMENT	42,935 C.Y.
SEEDING	17,583 S.Y.

300' V.C.
SSD EXCEEDS DESIGN
SPEED REQUIREMENTS

PVC STA. 155+50
ELEV.= 1052.02

PVI STA. 158+50
ELEV.= 1057.84

PVI STA. 157+00
ELEV.= 1053.34

PVC STA. 160+25
ELEV.= 1063.09

1350' V.C.
DESIGN SPEED = 64 mph
SSD = 547

14–D
97 L.F. 12"COND. TYPE D,
RT. ● 0.64%

13–D
102 L.F. 12"COND. TYPE D,
LT. ● 2.00%

15–D
93 L.F. 12"COND. TYPE D,
● 0.52%

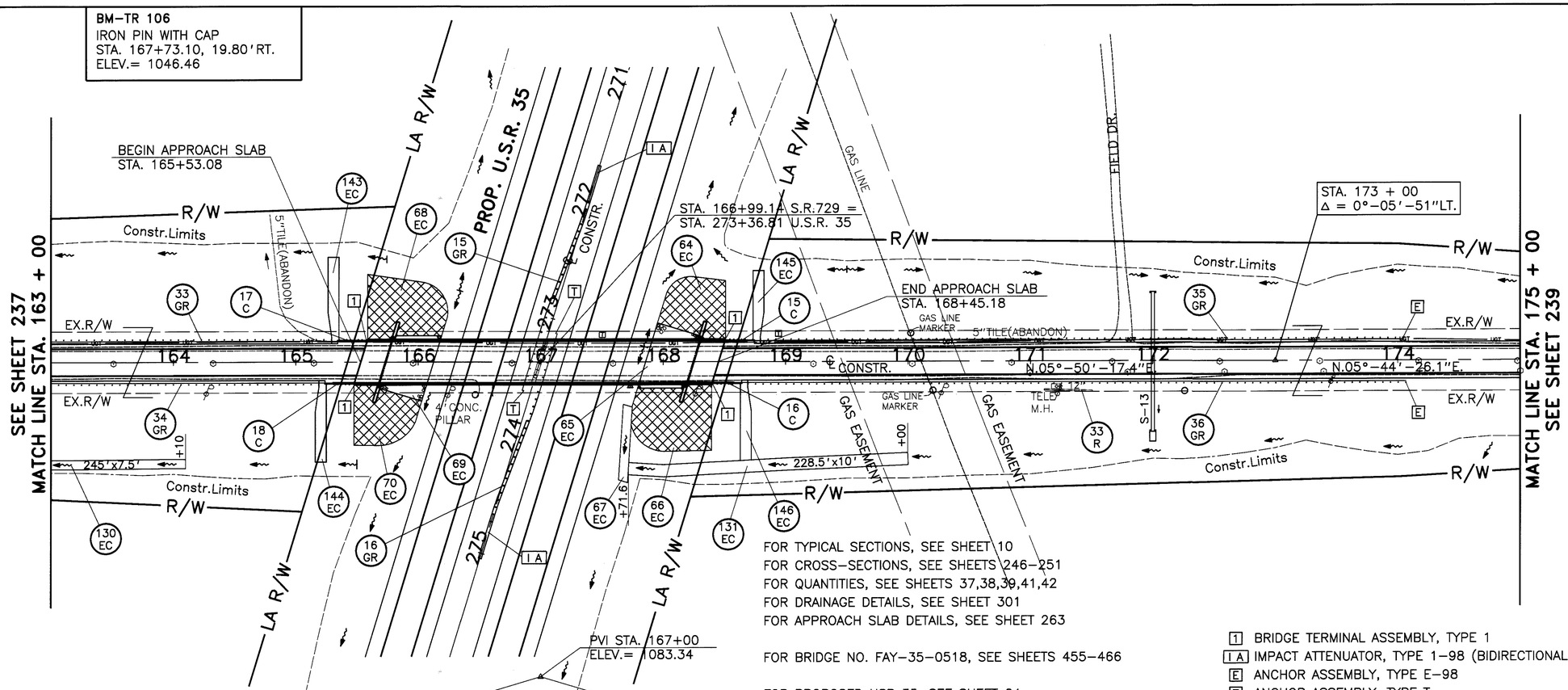
+0.76%
+0.88%
+3.00%
+3.00%

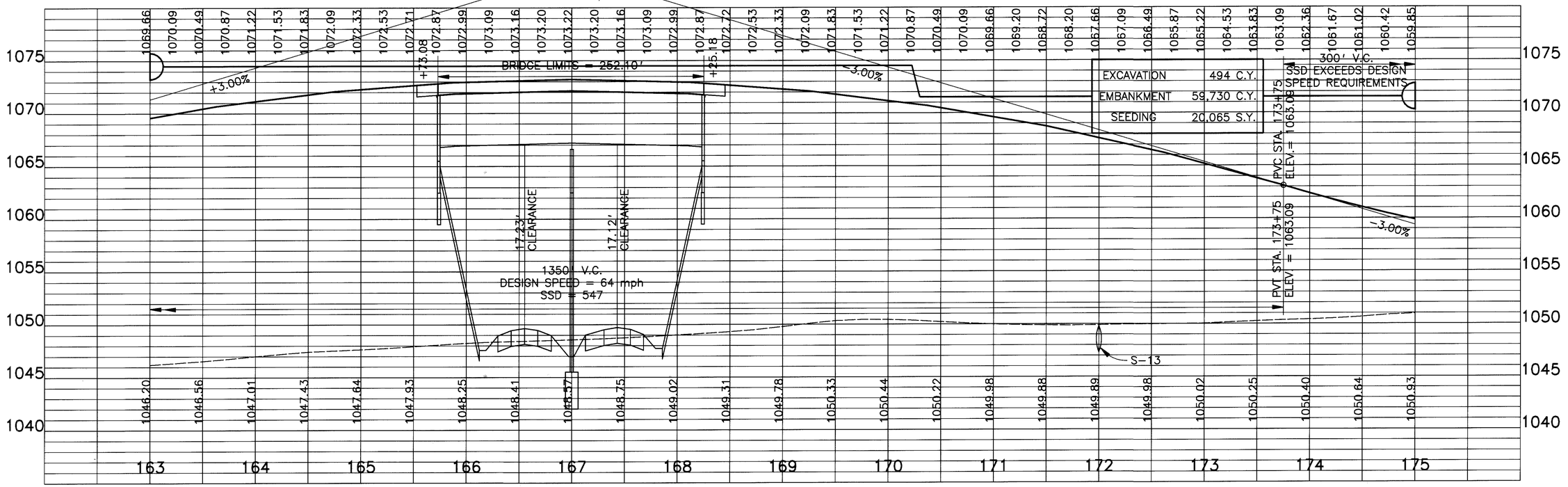
PLAN & PROFILE S.R. 729
STA. 151 + 00 TO STA. 163 + 00

FAY–35–2.57

237
522

BM–TR 106
IRON PIN WITH CAP
STA. 167+73.10, 19.80′ RT.
ELEV.= 1046.46





BM–TR 107
IRON PIN WITH CAP
STA. 180+69.88, 13.67' LT.
ELEV.= 1051.77

SEE SHEET 238
MATCH LINE STA. 175 + 00

+00 FIELD DR.

14 DW

16 D

PAVEMENT TAPER LT.

+50

+44

R/W

Constr.Limits

EX.R/W

UST

176 177 178 179 180 181 182

℄ CONSTR.

N05°–44'–26.9"E.

EX.R/W

Constr.Limits

R/W

END WORK
STA. 181+44

EX. U.S.R. 35

107 108 109 110 111

CONSTR.

STA. 182+17.85 S.R.729 =
STA. 108+81.56 EX. U.S.R. 35

.FOR TYPICAL SECTIONS, SEE SHEET 10
FOR CROSS–SECTIONS, SEE SHEETS 251–254
FOR QUANTITIES, SEE SHEETS 37,39,42

FOR DRIVE DETAILS, SEE SHEET 11

EXCAVATION	1592 C.Y.
EMBANKMENT	5113 C.Y.
SEEDING	7652 S.Y.

300' V.C.
SSD EXCEEDS DESIGN
SPEED REQUIREMENTS

PVI STA. 176+75
ELEV.= 1057.03

PVI STA. 180+50
ELEV.= 1053.13

PVI STA. 175+25
ELEV.= 1058.59

-3.00%
-1.04%
-1.04%
-1.20%
-1.16%

+44
+17.92

EX. U.S.R. 35

16–D
43 L.F. 12"COND. TYPE D.
● 1.60%

1059.85 1059.33 1058.84 1058.40 1057.99 1057.63 1057.31 1057.03 1056.77 1056.25 1055.73 1055.21 1054.69 1054.17 1053.65 1053.13 1052.53 1051.99

1050.93 1050.94 1050.96 1051.20 1051.61 1051.88 1052.20 1052.58 1053.00 1053.23 1053.15 1052.89 1052.38 1051.93 1051.48 1051.11

1075 1070 1065 1060 1055 1050 1045 1040

175 176 177 178 179 180 181 182 183

N

0 25 50 100
HORIZONTAL SCALE IN FEET

CALCULATED

CHECKED

PLAN & PROFILE S.R. 729
STA. 175 + 00 TO STA. 183 + 00

FAY–35–2.57

239
522

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
75		43	53			
	386 (-63) DEDUCT FOR DRIVE LT.			78	73	
				(0)	(3) ADD FOR DRIVE	
64		41	25			
	175			33	14	
62		29	5			
0						

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120 130

CONST.

1070 1060 1050 1040

PROP. R/W — EX. R/W — 10.36' — 11' — EX. R/W — PROP. R/W

4:1 — 4:1 — 1046.3 — 31.2' — UCT — 1047.4 — 27.4'

1050.26

STATION 153+50.00

1048.46

PROP. TEMP. R/W — PROP. R/W — EX. R/W — 9.69' — EX. R/W — 0.95%

6.0% — EX 12"

1049.93

STATION 153+13.00 DRIVE LT.

1048.67

PROP. R/W — EX. R/W — 4' — 9.45' — 11' — 4' — EX. R/W — PROP. R/W

4:1 — 4:1 — 1046.6 — 27.4' — EX 12" — 1047.1 — 26.9'

1049.82

STATION 153+00.00

1048.68

PROP. R/W — EX. R/W — 3' — 9' — 11' — 3' — EX. R/W — PROP. R/W

4:1 — 4:1

1049.60

STATION 152+75.00

1048.60

BEGIN WORK +75

PROP. R/W — EX. R/W — EX. R/W — PROP. R/W

UCT

STATION 152+50.00

1048.41

℄ CONST.

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120 130 140

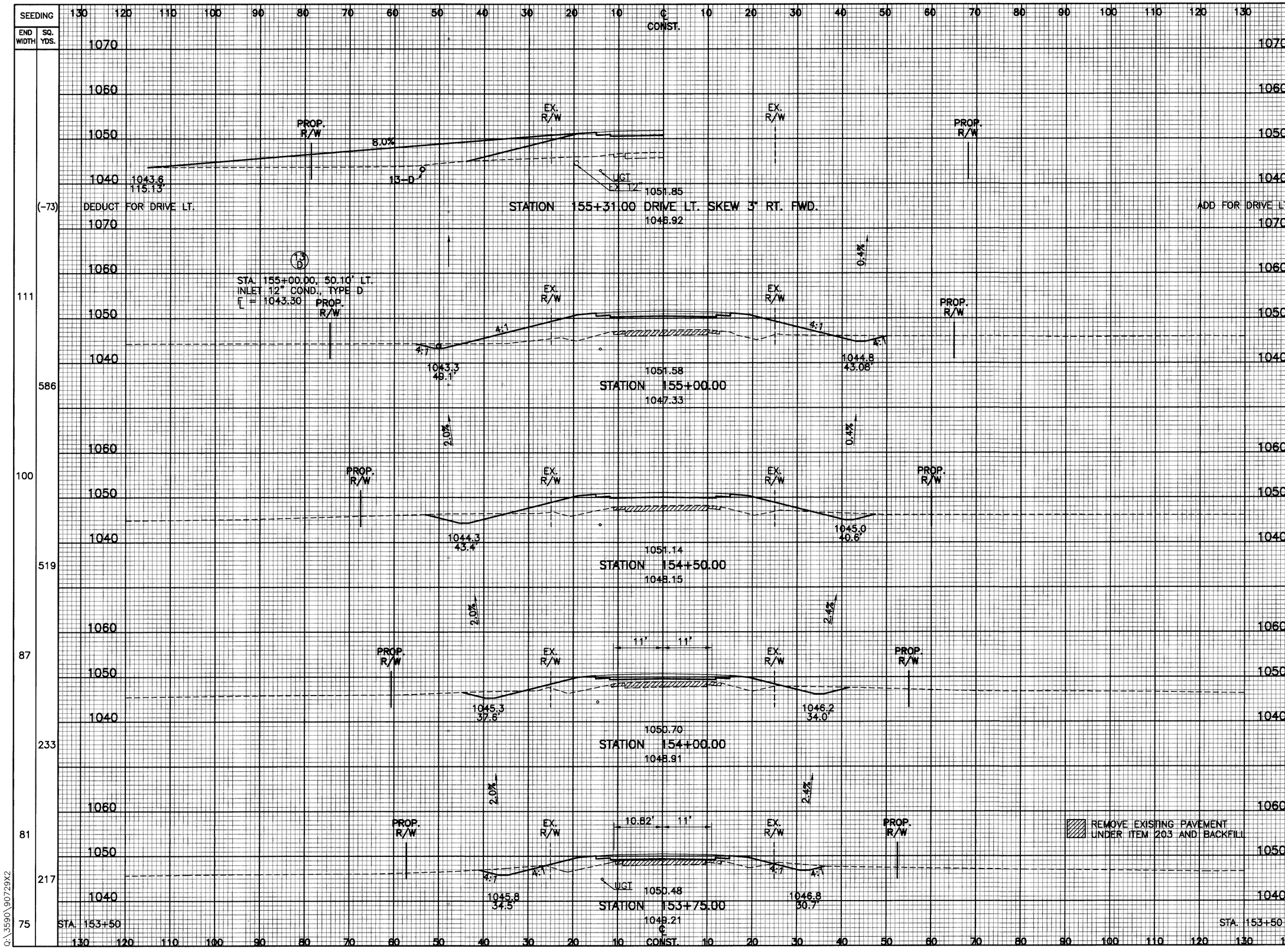
REMOVE EXISTING PAVEMENT UNDER ITEM 203 AND BACKFILL

Q:\3590\90729x1

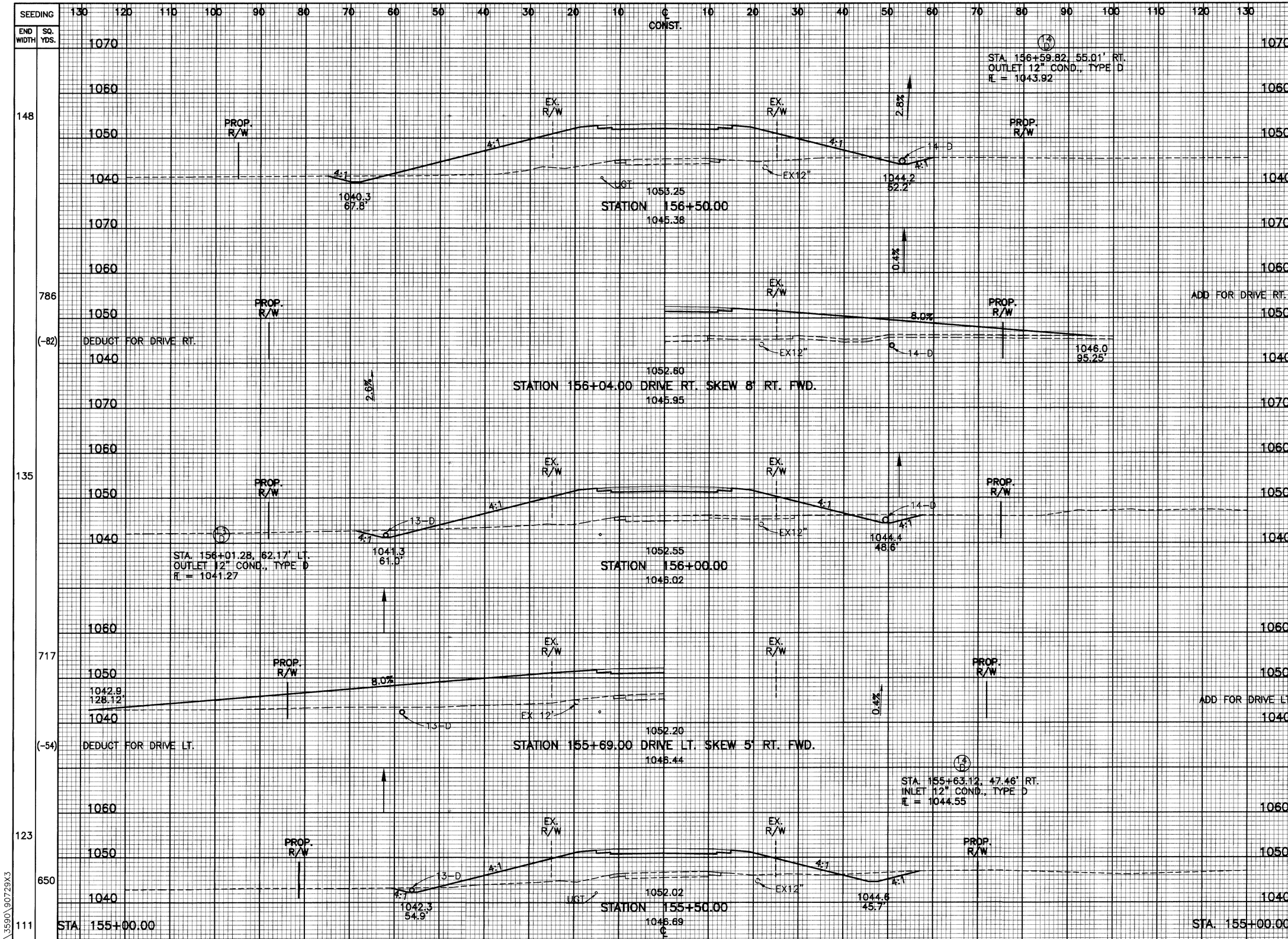
S.R. 729 X-SECTIONS STATION 152+50 TO STATION 153+50

FAY-35-2.57

240
522



SEEDING
END WIDTH
SQ. YDS.
END AREA
CUT
FILL
VOLUME
CUT
FILL
CALCULATED J.L.G.
CHECKED W.G.Y.
℄ CONST.
PROP. R/W
EX. R/W
8.0%
1043.6
115.13'
13-D
UGT
EX. 12"
1051.85
DEDUCT FOR DRIVE LT.
STATION 155+31.00 DRIVE LT. SKEW 3° RT. FWD.
1046.92
ADD FOR DRIVE LT.
(-73)
(0)
(236)
STA. 155+00.00, 50.10' LT.
INLET 12" COND., TYPE D
FL = 1043.30
0.4%
4:1
1043.3
49.1'
1044.8
43.08'
1051.58
STATION 155+00.00
1047.33
111
586
2.0%
0.4%
1044.3
43.4'
1045.0
40.6'
1051.14
STATION 154+50.00
1048.15
100
519
2.0%
2.4%
11'
11'
1045.3
37.6'
1046.2
34.0'
1050.70
STATION 154+00.00
1048.91
87
233
2.0%
2.4%
10.82'
11'
REMOVE EXISTING PAVEMENT UNDER ITEM 203 AND BACKFILL
1045.8
34.5'
1046.8
30.7'
1050.48
STATION 153+75.00
1049.21
81
217
75
STA. 153+50
STA. 153+50
41
283
58
177
50
104
48
74
43
53
92
426
100
261
46
83
43
59
S.R. 729 X-SECTIONS STATION 153+75 TO STATION 155+31
FAY-35-2.57
241
522
Q:\3590\90729X2

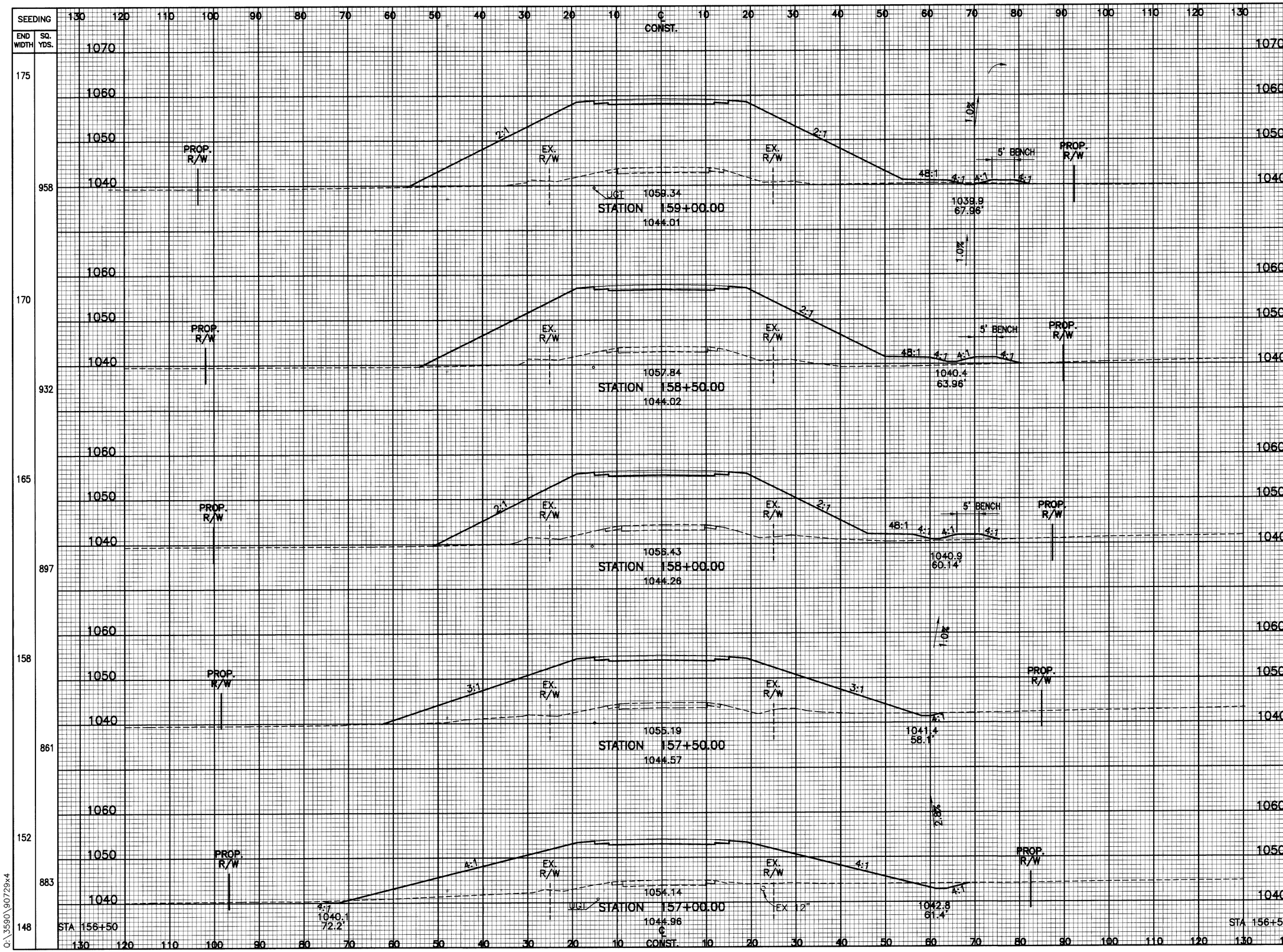


CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 155+50 TO STATION 156+50

FAY-35-2.57

Q:\3590\90729X3



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
4	1243		
		4	2188
0	1078		
		3	1885
3	916		
		6	1660
3	876		
		13	1527
11	773		
		29	1255
20	582		

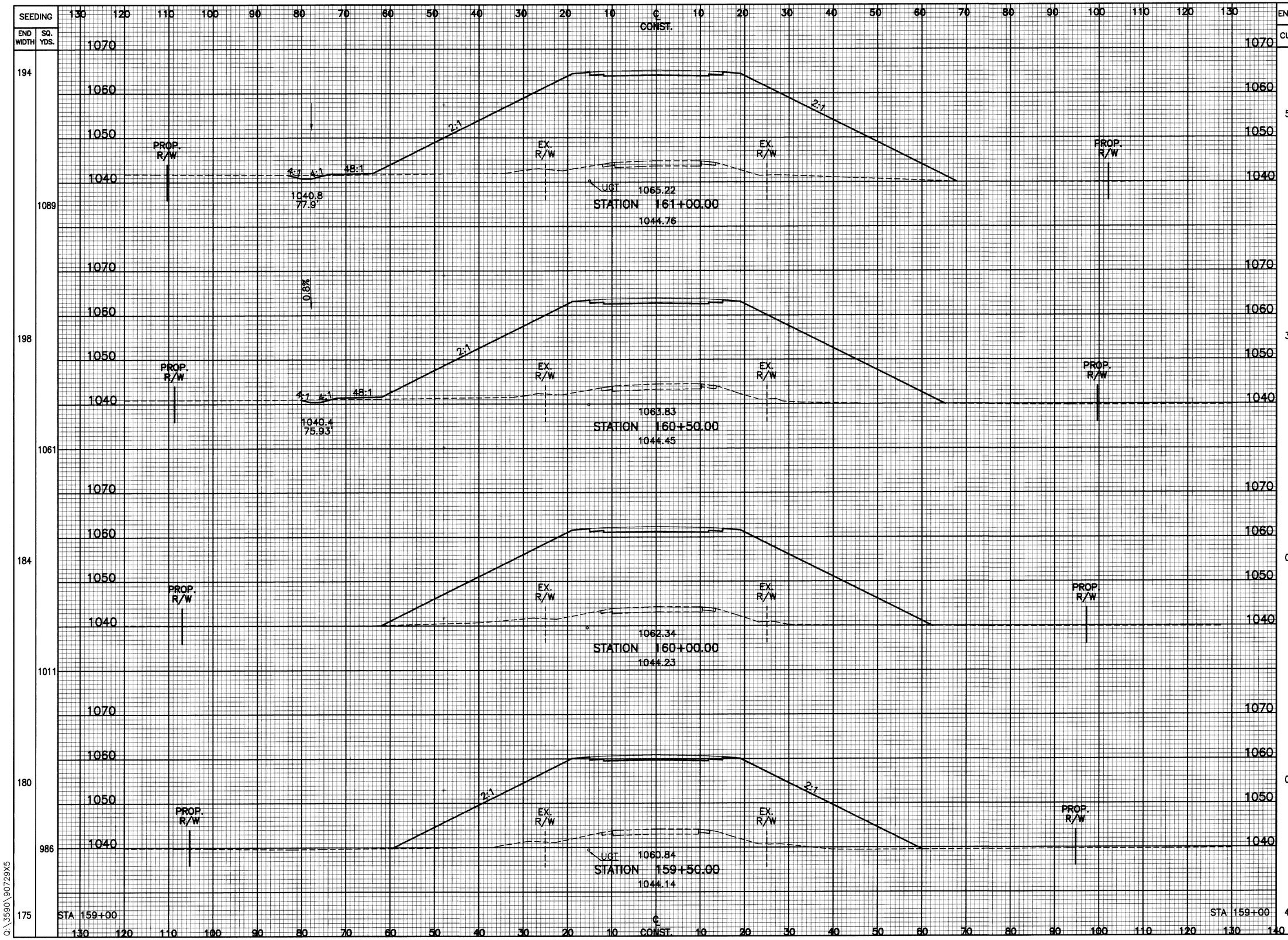
CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 157+00 TO STATION 159+00

FAY-35-2.57

Q:\3590\90729x4

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
194	1089	5	1812	8	3256	
198	1061	3	1704	3	3017	
184	1011	0	1554	0	2744	
180	986	0	1409	4	2456	
175		4	1243			



S.R. 729 X-SECTIONS STATION 159+50 TO STATION 161+00

FAY-35-2.57

244
522

Q:\3590\90729X5

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	CHECKED W.G.Y.
207		27	2047			
	1139			(1)	4325	
	(+402) ADD FOR DRIVE LT.			47	3708	
203		23	1957			
	1114			30	3588	
198		9	1918			
	1089			13	3454	
194		5	1812			

STA. 162+67.02, 84.41' LT.
INLET 12" COND., TYPE D
℄ = 1041.87

PROP. R/W
EX. R/W
15-D
2:1
4:1 4:1 48:1
1041.8
82.9'
UGT 1068.72
STATION 162+50.00
1045.83
1042.0
82.5'
48:1 4:1 4:1
℄ CONST.

1043.0
232.5'
12.0%
ADD FOR DRIVE LT.
STATION 162+50 DRIVE LT.
SKEW 20° RT. FWD.
1045.83

0.4%
2.0%
TEMP R/W @ 162+00.00, 235.00' LT.
1041.8
81.2'
1067.66
STATION 162+00.00
1045.44
1041.0
82.4'

STA. 161+74.06, 81.42' LT.
OUTLET 12" COND., TYPE D
℄ = 1041.39

0.8%
1.2%
TEMP R/W @ 161+50.00, 235.0000' LT.
TEMP R/W @ 161+50.00, 208.8507' LT.
1041.2
79.7'
UGT 1066.49
STATION 161+50.00
1045.12
1040.4
81.3'

STA. 161+00

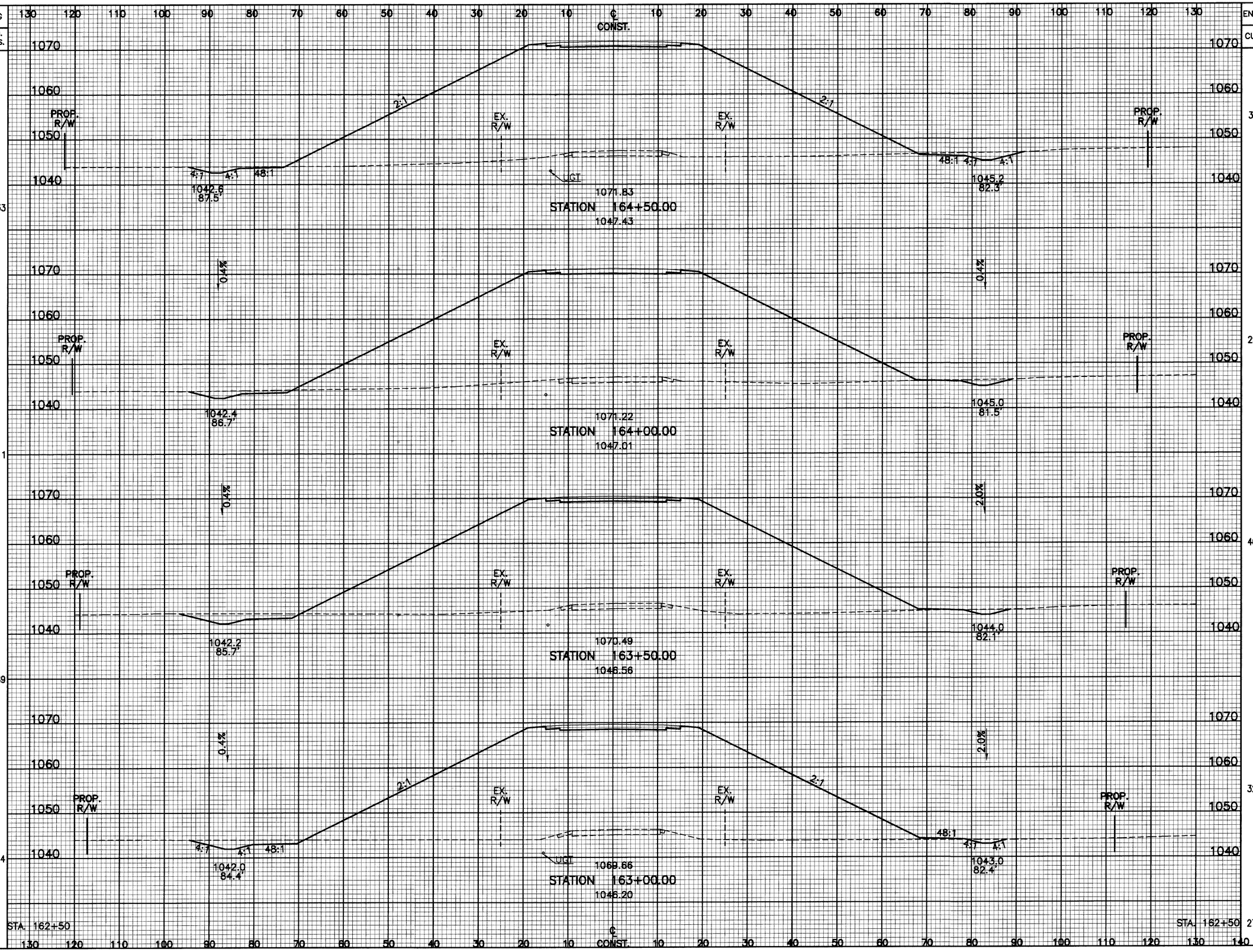
S.R. 729 X-SECTIONS STATION 161+50 TO STATION 162+50

FAY-35-2.57

245
522

Q:\3590\90729X6

SEEDING	
END WIDTH	SQ. YDS.
224	1233
220	1211
216	1189
212	1164
207	



END AREA		VOLUME	
CUT	FILL	CUT	FILL
31	2236	56	4077
29	2167	64	4018
40	2172	67	3984
32	2130	55	3868
27	2047		

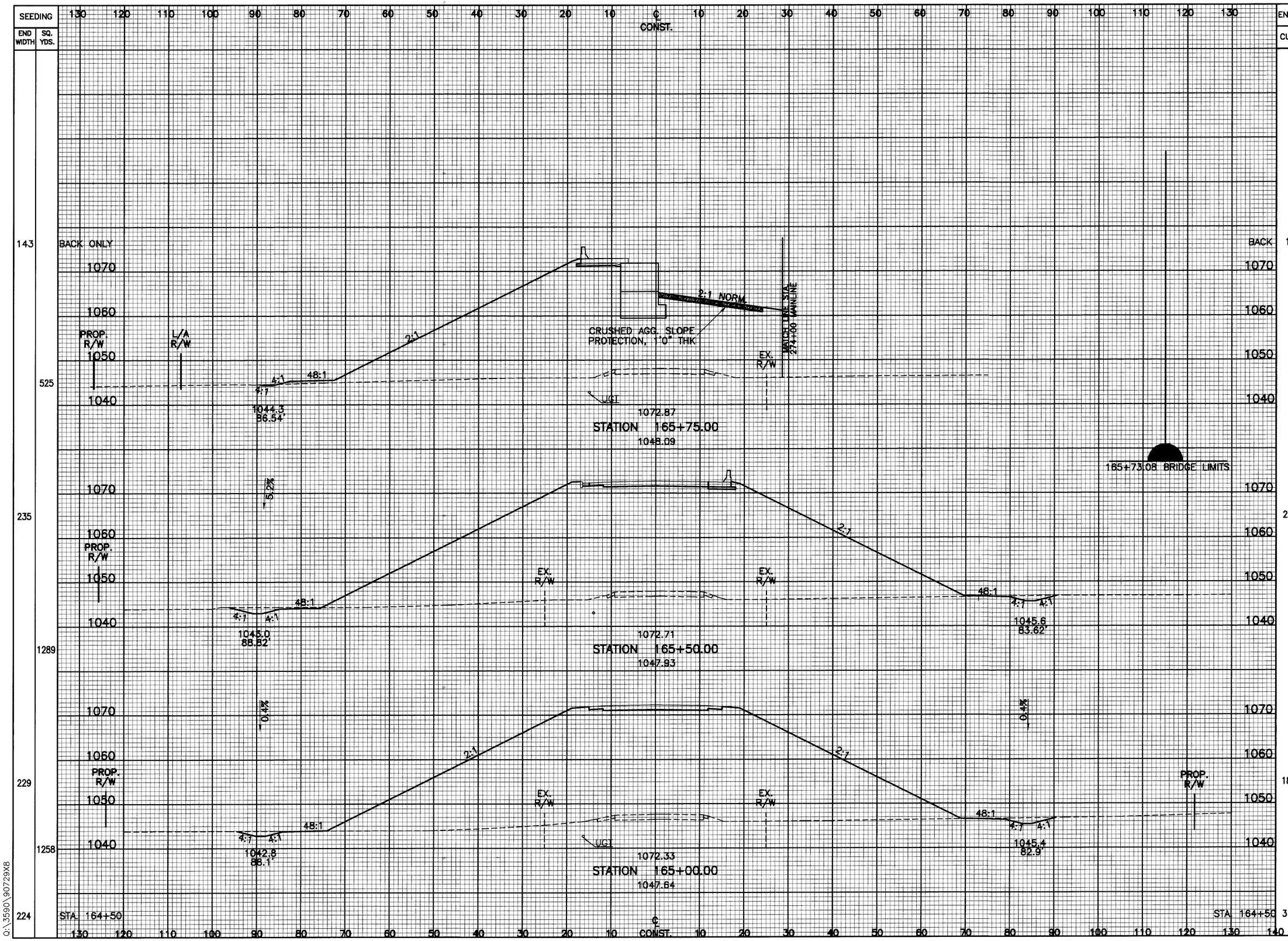
CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 163+00 TO STATION 164+50

FAY-35-2.57

246
522

Q:\3590\90729x7



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
1	1597		
		11	1830
21	2354		
		37	4314
18	2305		
		46	4205
31	2236		

CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 165+00 TO STATION 165+75

FAY-35-2.57

Q:\3590\90729X8

SEEDING END WIDTH	SEEDING SQ. YDS.
176	975
175	503
155	290
54	

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
6	1938		
		16	3782
11	2146		
		7	1858
AHD. 4	1867		
BACK 4	1149		
		6	608
7	163		

CALCULATED J.L.G.
CHECKED W.C.Y.

1072.33
STATION 169+00.00
1049.78

1072.72
STATION 168+50.00
1049.31

1072.87
STATION 168+25.00
1049.15

1072.99
STATION 168+00.00
1049.02

1048.5 76.7'
1046.6 85.0'
1045.7 82.34'
1045.2 85.12'
1044.7 86.42'
1044.45 87.66'

168+25.18 BRIDGE LIMITS

MATCH LINE STA. 272+50 MAINLINE
MATCH LINE STA. 273+50 MAINLINE
MATCH LINE STA. 274+00 MAINLINE

CRUSHED AGG. SLOPE PROTECTION, 1'0" THK

S.R. 729 X-SECTIONS STATION 168+00 TO STATION 169+00

FAY-35-2.57

248
522

Q:\3590\90729X9

SEEDING END WIDTH	SEEDING SQ. YDS.
181	1014
184	1025
185	1033
187	1008
176	

STATION 171+00.00
1069.96
1048.98
PROP. R/W
EX. R/W
EX. R/W
2:1
2:1
0.2%
48:1
4:1
4:1
4:1
5' BENCH
1048.2
72.6'
1046.9
75.2'
UGT

STATION 170+50.00
1070.49
1050.22
0.8%
0.2%
1048.6
72.9'
1046.8
76.46
26" GAS LINE

STATION 170+00.00
1071.22
1050.44
0.8%
1.0%
1049.0
73.5'
1046.7
75.12

STATION 169+50.00
1071.83
1050.33
0.6%
1.6%
1.0%
1049.3
74.1'
1046.2
80.34

STA. 169+00

END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
8	1665	18	3082
11	1663	22	3100
12	1684	22	3172
11	1741	16	3407
6	1938		

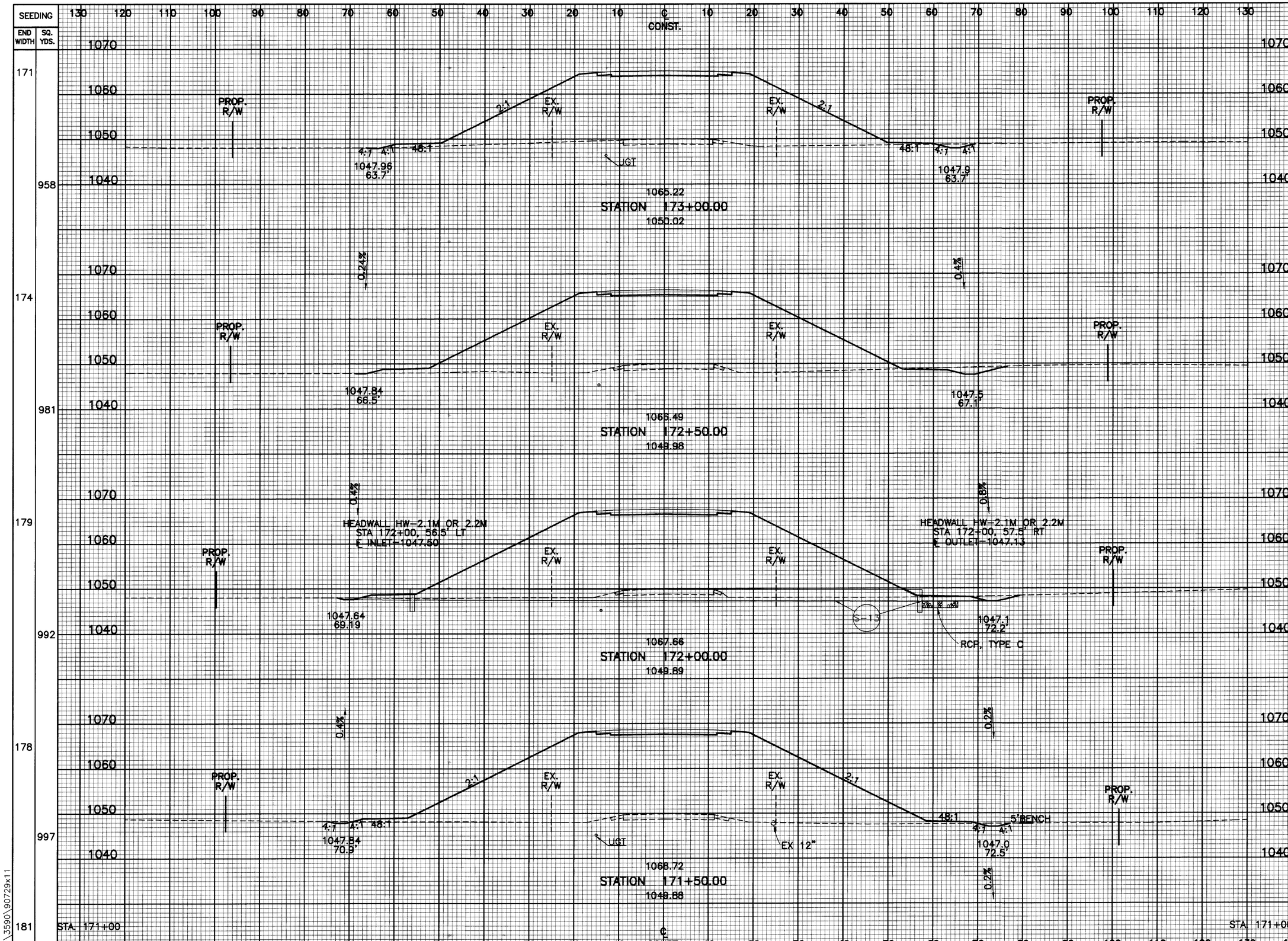
CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 169+50 TO STATION 171+00

FAY-35-2.57

249
522

Q:\3590\90729X10



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
5	1062		
		21	2130
17	1238		
		25	2437
9	1393		
		13	2698
5	1520		
		13	2950
8	1665		

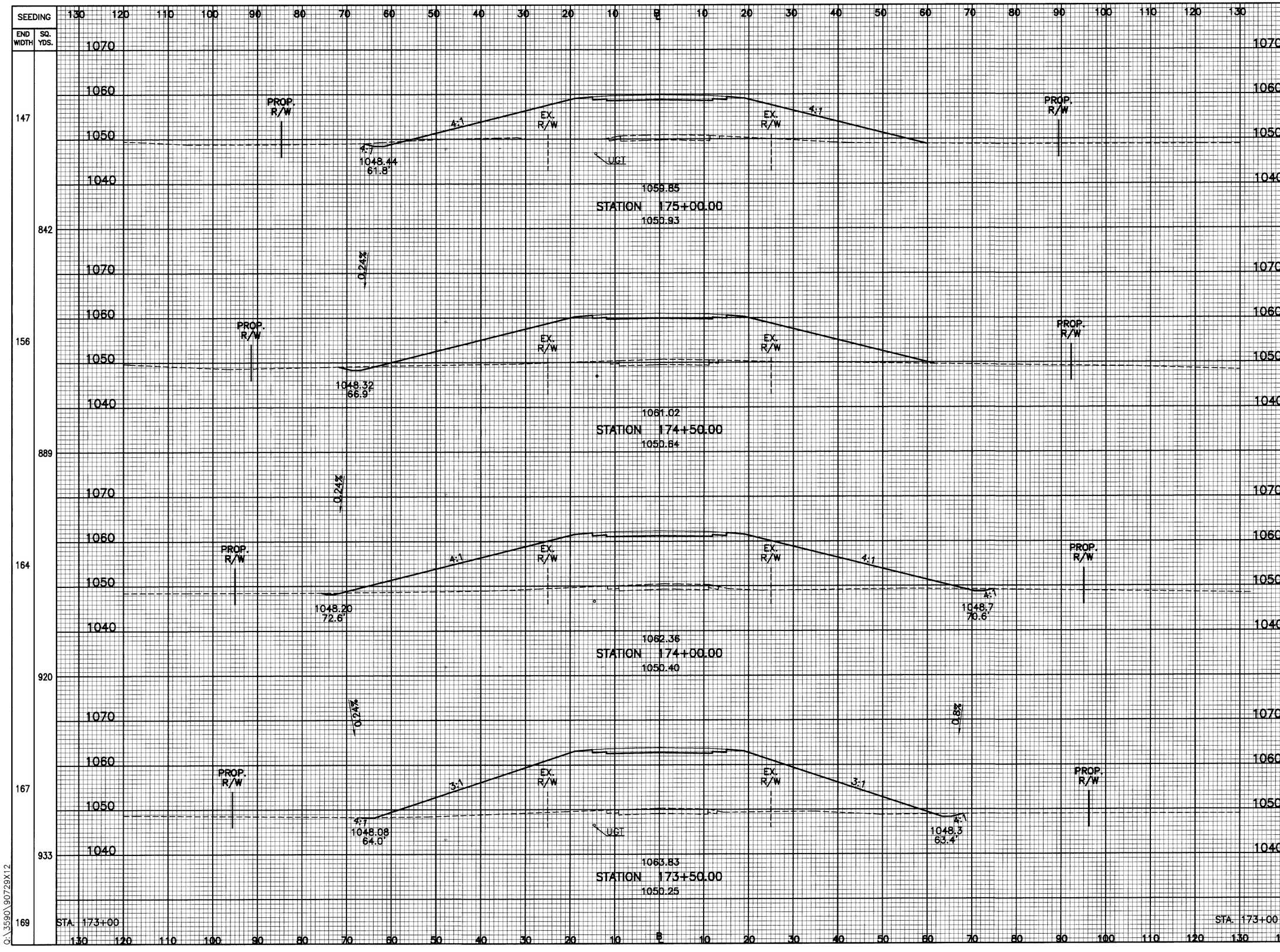
CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 171+50 TO STATION 173+00

FAY-35-2.57

250
522

Q:\3590\90729x11



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
5	667		
		10	1366
5	808		
		8	1732
3	1062		
		7	1990
4	1087		
		9	1990
5	1062		

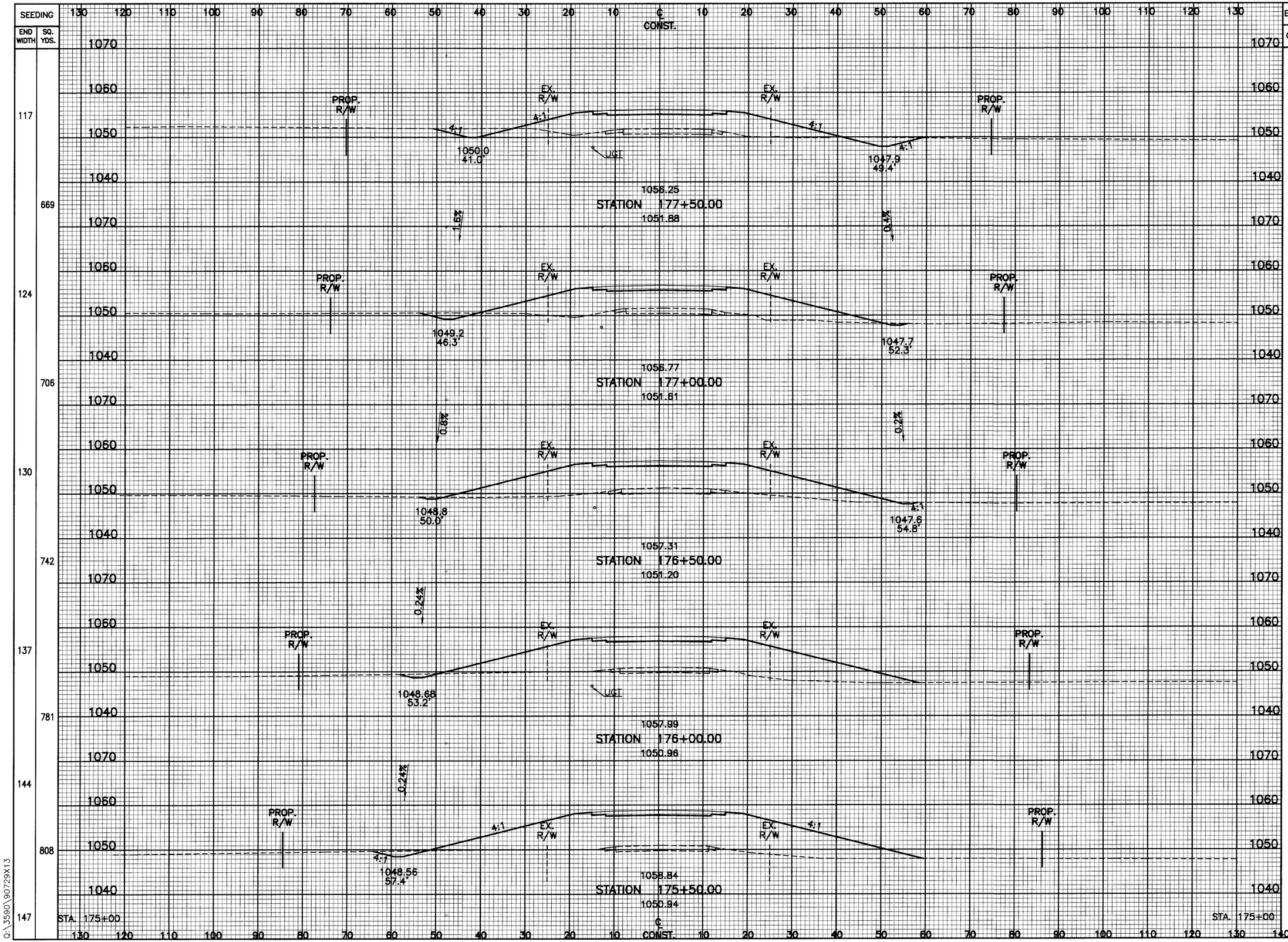
CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 173+50 TO STATION 175+00

FAY-35-2.57

Q:\3590\90729X12

SEEDING		END AREA		VOLUME		CALCULATED J.L.G.	CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL		
117	669	39	237	48	543		
124	706	12	349	14	645		
130	742	3	347	8	812		
137	781	5	529	13	1048		
144	808	9	602	13	1175		
147		5	667				

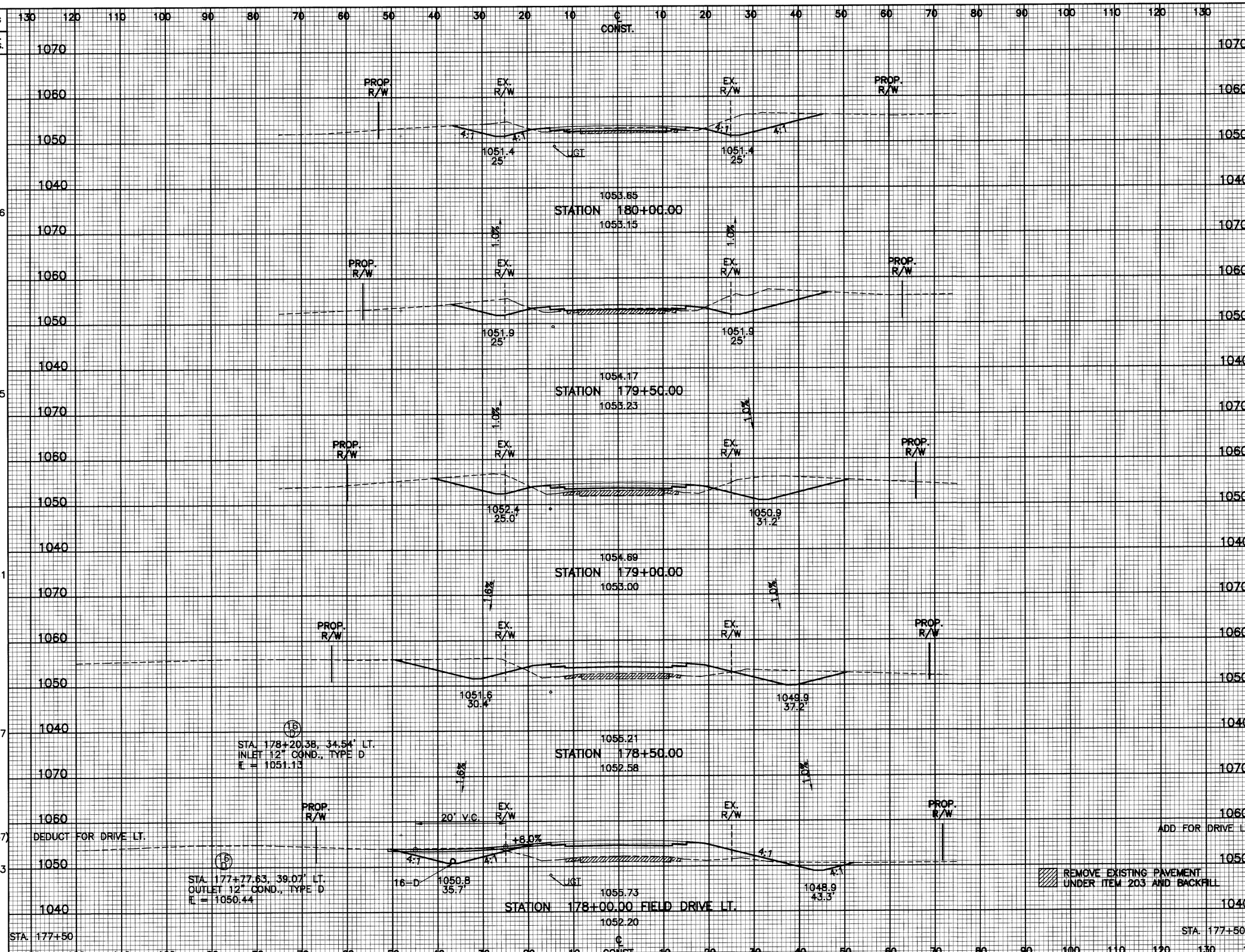


S.R. 729 X-SECTIONS STATION 175+50 TO STATION 177+50

FAY-35-2.57

252
522

Q:\3590\90729X13



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
119	15		
		233	41
132	29		
		273	83
162	60		
		285	155
145	107		
		219	231
91	142	(1)	(1)
		121	351
39	237		

CALCULATED J.L.G.
CHECKED W.G.Y.

S.R. 729 X-SECTIONS STATION 178+00 TO STATION 180+00

FAY-35-2.57

253/522

Q:\3590\90729X14

SEEDING	
END WIDTH	SQ. YDS.
67	342
73	419
78	450
84	

END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
CUT	FILL	CUT	FILL	
14	0			
		50	3	
47	3			
		124	7	
86	4			
		190	18	
119	15			

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130

1060
1050
1040
1070

UGT

STATION 181+75.00
1051.64

PROP. R/W
EX. R/W
EX. R/W
PROP. R/W

4:1
4:1
4:1
4:1

1049.6
22.1'

1049.7
20.7'

1051.89
STATION 181+44.00
1051.99

END WORK +44

1.8%
1.4%

1050.3
26.4'

1050.3
25'

1052.53
STATION 181+00.00
1052.38

1.2%
0.8%

1050.9
25'

1050.9
25'

LGT

1053.13
STATION 180+50.00
1052.89

REMOVE EXISTING PAVEMENT UNDER ITEM 203 AND BACKFILL

STA. 180+00

STA. 180+00

℄ CONST.

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ CONST. 10 20 30 40 50 60 70 80 90 100 110 120 130 140

Q:\3590\90729X15

S.R. 729 X-SECTIONS STATION 180+50 TO STATION 181+75

FAY-35-2.57

254
522

SEEDING	
END WIDTH	SQ. YDS.
89	225
73	163
44	124
45	81
13	

END AREA		VOLUME	
CUT	FILL	CUT	FILL
2	144	17	73
33	12	57	10
88	9	50	9
20	9	10	5
0	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

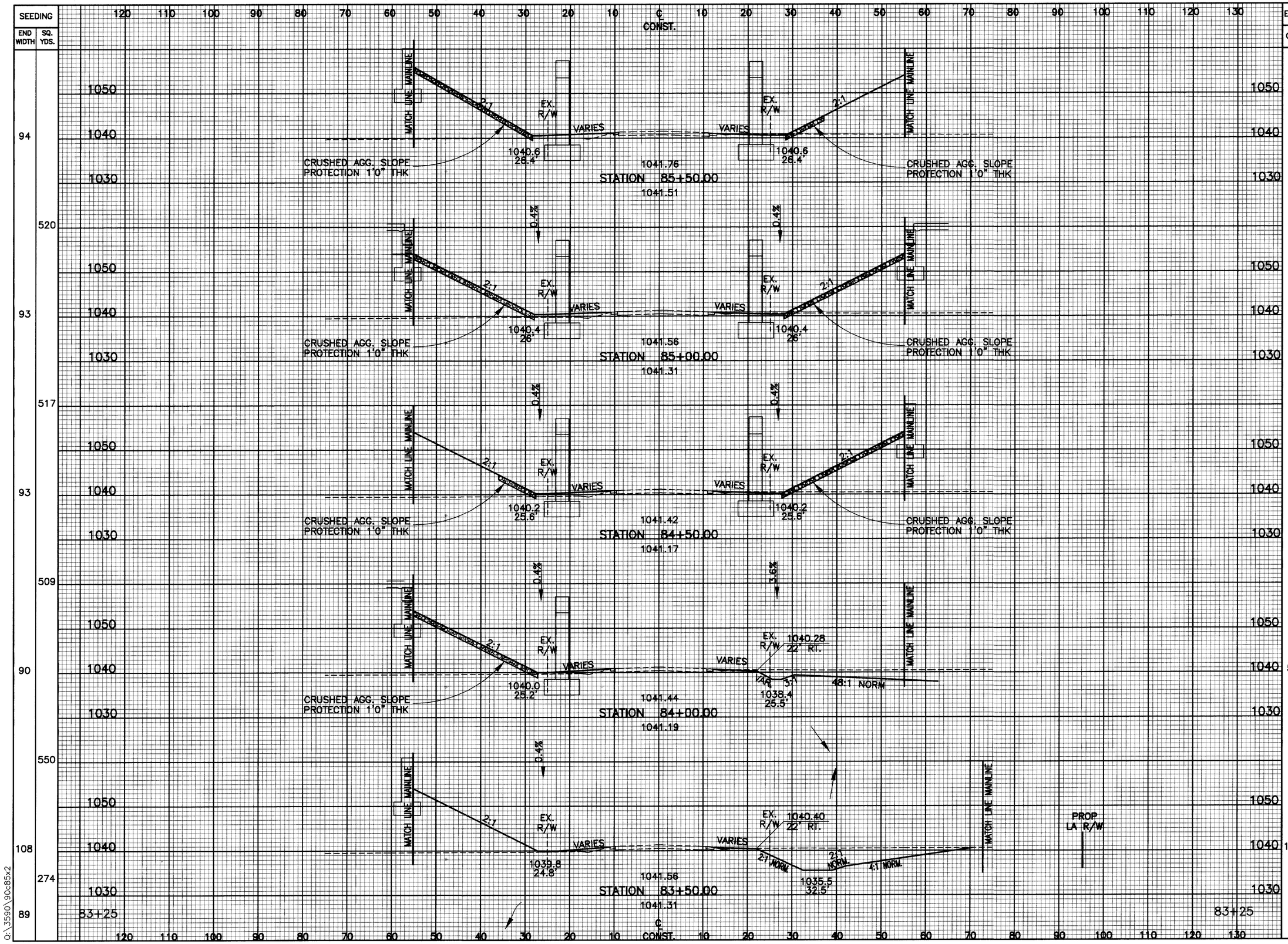
C.R 85 X-SECTIONS STATION 82+00 TO STATION 83+25

FAY-35-2.57

255
522

Q:\3590\90c85x1

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
94	520	7	414	10	756	
93	517	3	402	7	755	
93	509	4	413	56	587	
90	550	56	220	172	400	
108	274	129	212	61	165	
89		2	144			



C.R 85 X-SECTIONS STATION 83+50 TO STATION 85+50

FAY-35-2.57

256
522

O:\3590\90c85x2

SEEDING	
END WIDTH	SQ. YDS.
103	313
122	314
104	270
90	249
89	509
94	

END AREA		VOLUME	
CUT	FILL	CUT	FILL
113	6	63	40
21	80	107	107
210	149	154	140
122	152	88	150
66	170	68	541
7	414		

CALCULATED J.L.G.
CHECKED W.G.Y.

HEADWALL HW-2.1M OR 2.2M
STA. 87+22.23, 38.99' RT.
E. OUTLET = 1033.94

HEADWALL HW-2.1M OR 2.2M
STA. 86+54.59, 34.58' LT.
E. INLET = 1034.18

STATION 87+00.00
STATION 86+75.00
STATION 86+50.00
STATION 86+25.00
STATION 86+00.00

CRUSHED AGG. SLOPE PROTECTION 1'0" THK

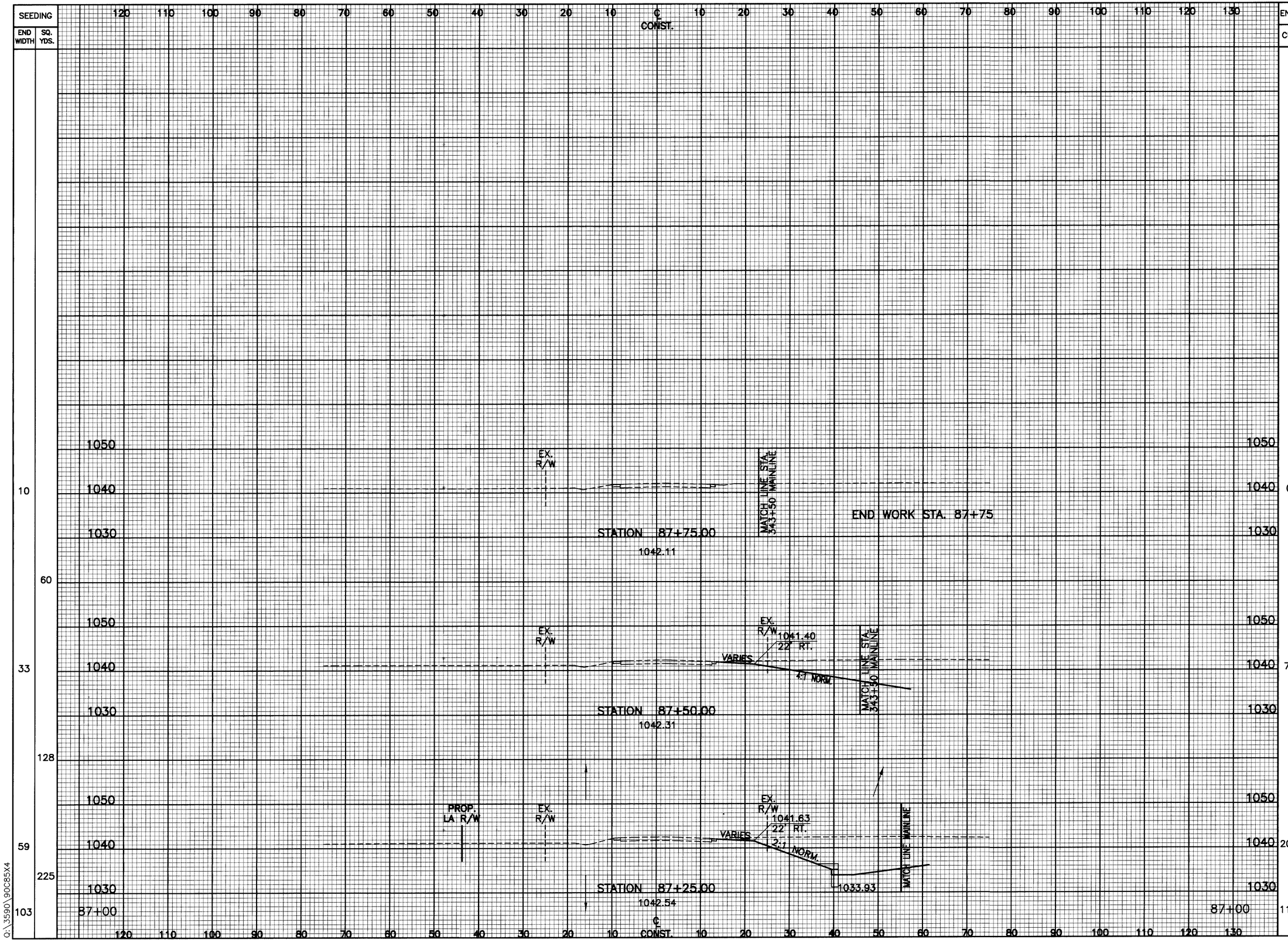
C.R 85 X-SECTIONS STATION 86+00 TO STATION 87+00

FAY-35-2.57

257
522

Q:\3590\90C85X3

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
10	60	0	0	33	0	
33	128	71	0	130	0	
59	225	209	0	150	3	
103		113	6			

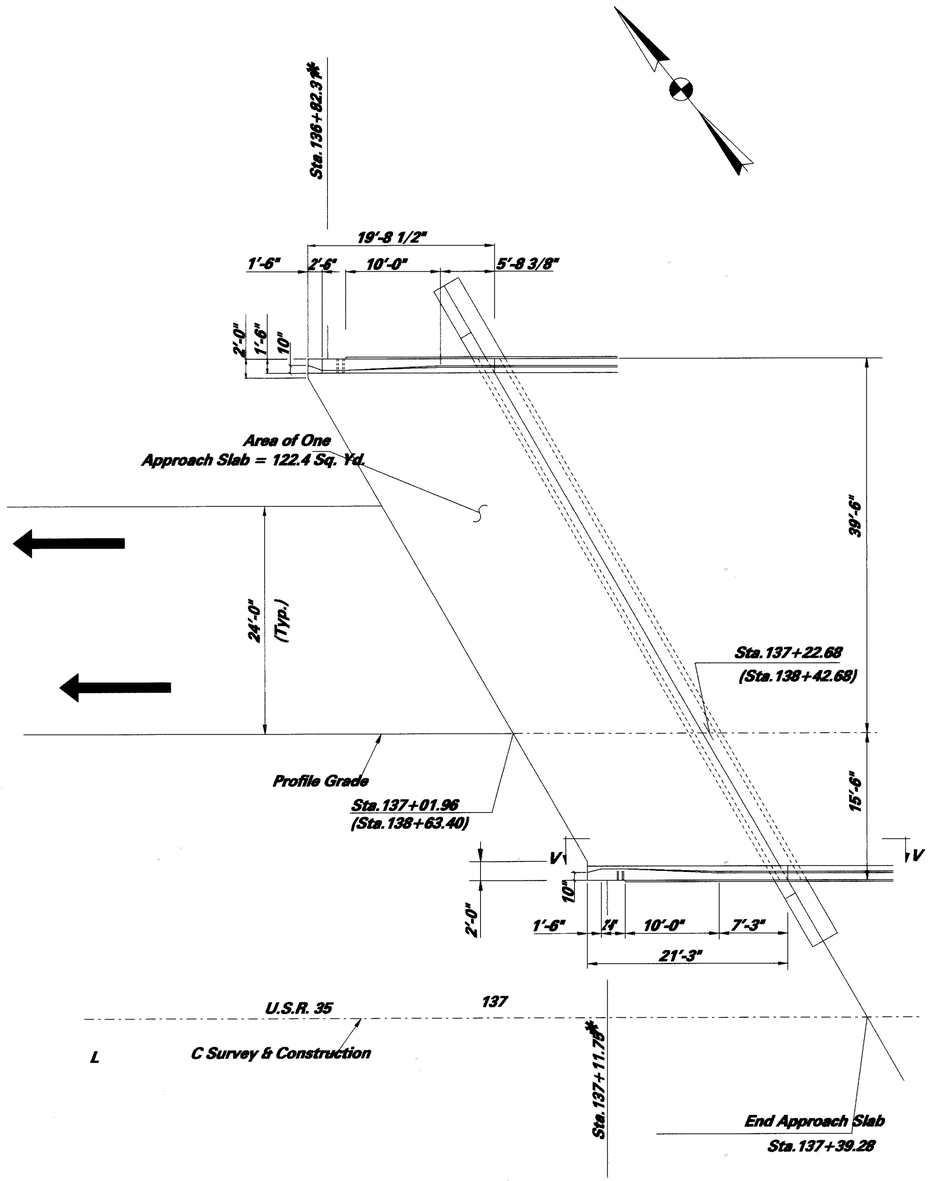


C.R 85 X-SECTIONS STATION 87+25 TO STATION 87+75

FAY-35-2.57

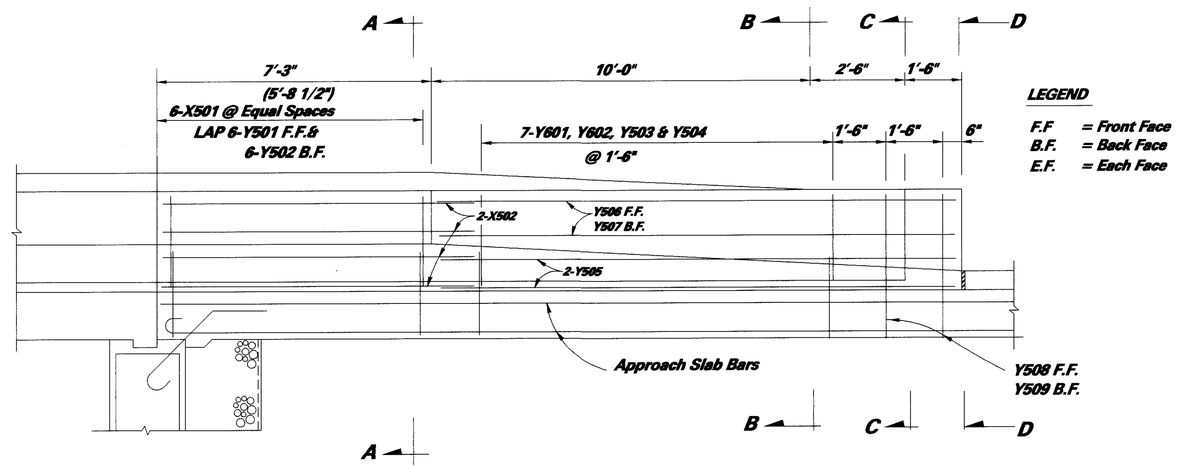
258
522

Q:\3590\90C85X4

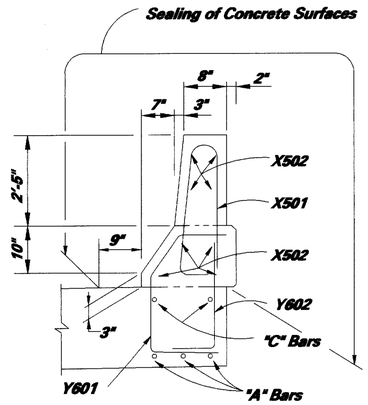


Rear Approach Slab - Left Bridge

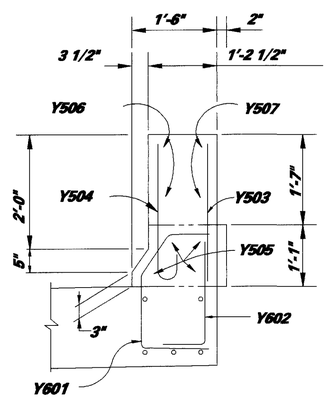
Similar At 3 Other Locations



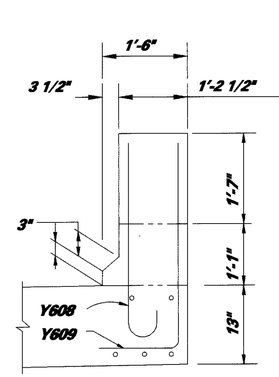
VIEW V-V



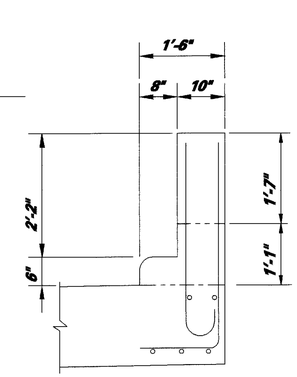
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

Stations in ( ) are for Forward Approach Slab.

The portion of the approach slab which extends under the parapet shall be included in the cost of Item 611.

All parapet concrete including transition is HPC.

REINFORCING STEEL LIST (ONE BRIDGE)

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	52	3-6	273	5	1-1 1/2	
Y602	52	2-9	215	4	0-10 1/2	2-0
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	7		
Y507	8	13-8	114	STR		
Y608	8	3-10	46	8	3-3	
Y609	8	4-10	54	4	1-8 1/2	3-3
X501	24	7-1	177	6		
X502	32	9-3	309	STR		
		TOTAL	1683	LBS. X 2 Bridges = 3366 Lbs.		

For Bar Bending Diagram See Sheet 403/522

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

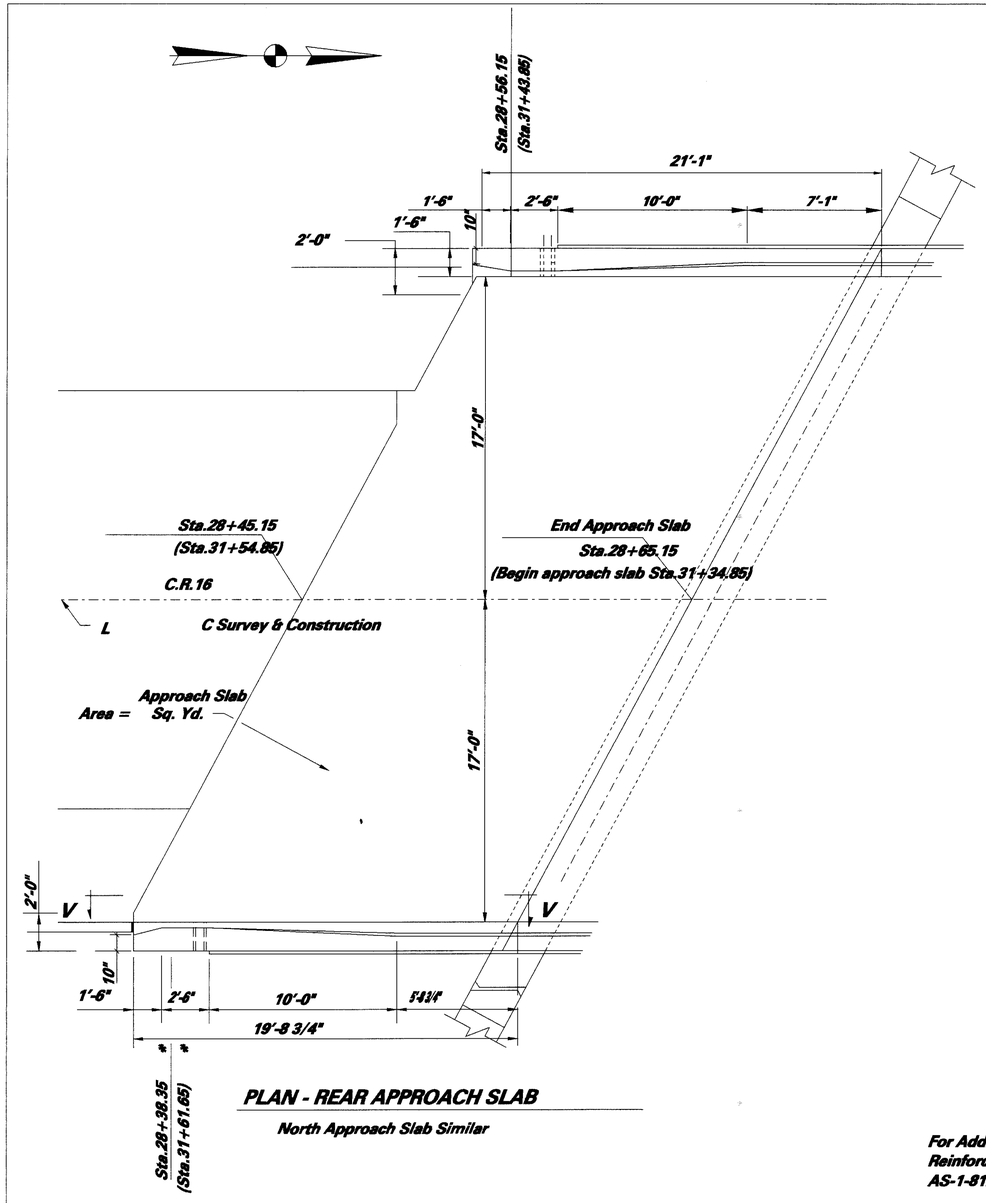
DESIGNED SP
DRAWN SP
CHECKED RT
REVISED
REVIEWED
DATE

STRUCTURE FILE NO. 2400389

APPROACH SLAB DETAILS

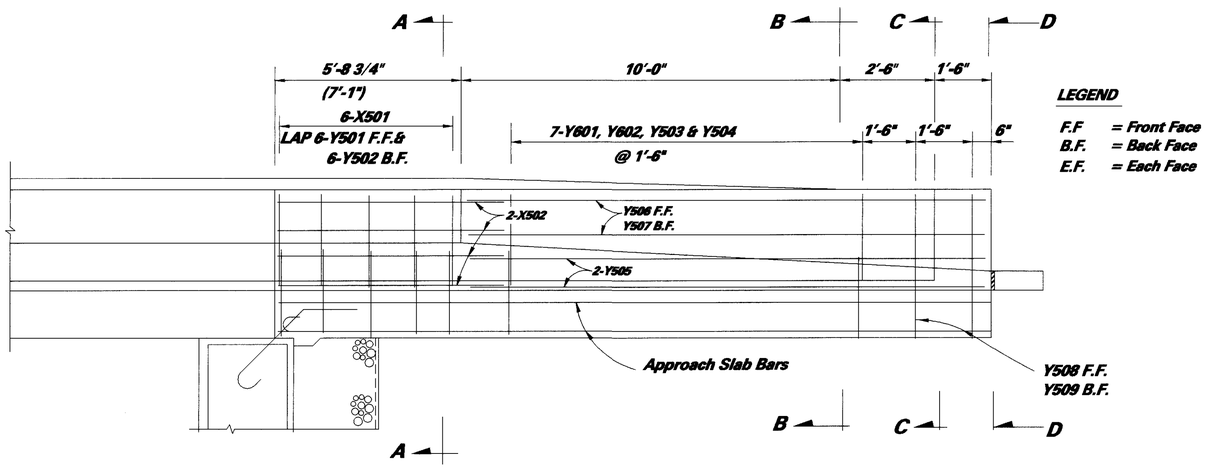
BRIDGE NO. FAY-35-0259 L & R.
OVER RATTLE SNAKE CREEK

FAY-35-2.57

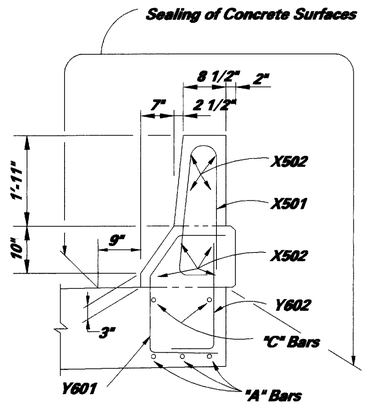


PLAN - REAR APPROACH SLAB

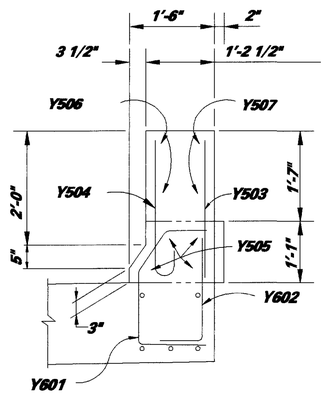
North Approach Slab Similar



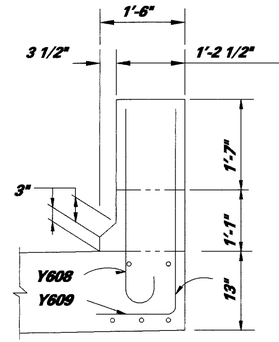
VIEW V-V



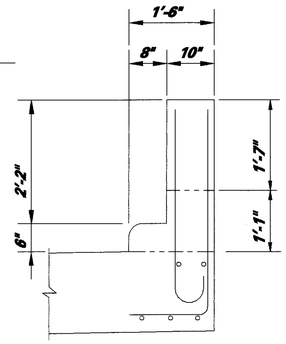
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

Stations in ( ) are for Forward Approach Slab.

The portion of the approach slab which extends under the parapet shall be included in the cost of Item 611.

All parapet concrete including transition is HPC.

REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	52	3-5	267	5	1-0	
Y602	52	2-6	195	4	0-10 1/2	1-9
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	7		
Y507	8	13-8	114	STR		
Y608	8	3-10	46	8	3-3	
Y609	8	4-6	54	4	1-4 1/2	3-3
X601	24	6-0	150	6		
X502	32	8-6	284	STR		
		TOTAL	1605	LBS.		

For Bar Bending Diagram See Sheet 416 522

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

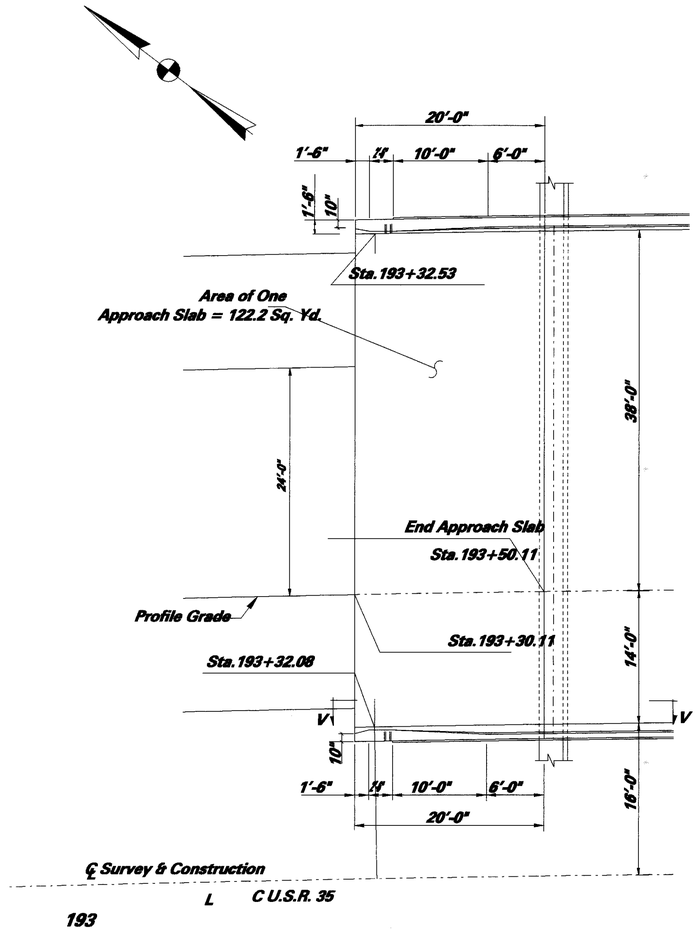
DESIGNED RT
DRAWN RT
CHECKED VK
REVIEWED
DATE
REVISED

STRUCTURE FILE NO.2400324

APPROACH SLAB DETAILS

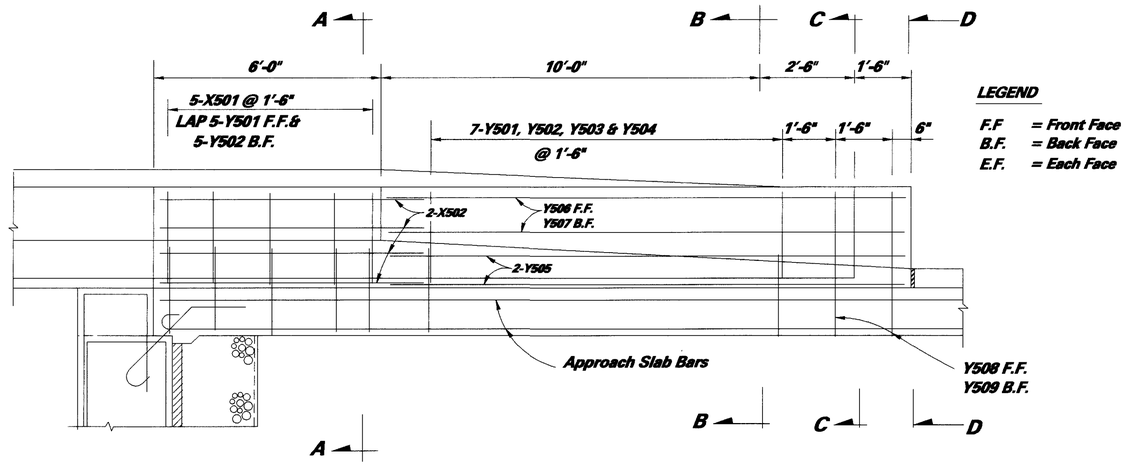
BRIDGE NO. FAY-35-0308
C.R.16 OVER US 35

FAY-35-2.57

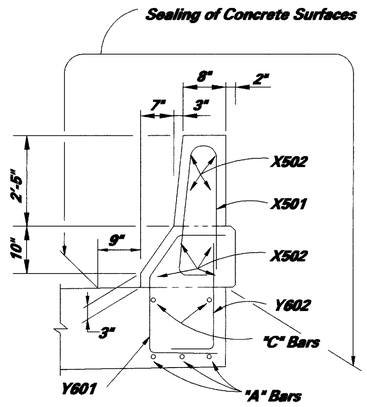


Rear Approach Slab - Left Bridge

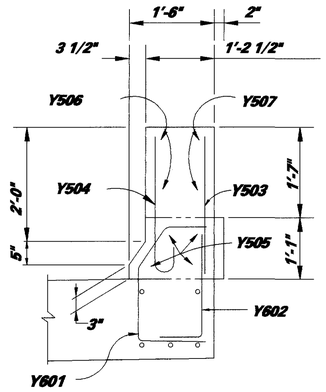
Similar At 3 Other Locations



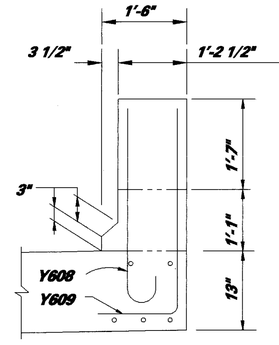
VIEW V-V



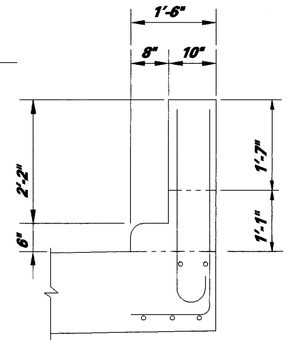
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

The portion of the approach slab which extends under the parapet shall be included in the cost of item 611.

All paraper concrete including transition is HPC

REINFORCING STEEL LIST (ONE BRIDGE)

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	48	3-5	246	6	1-0	
Y602	48	2-6	180	5	0-10 1/2	1-9
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	9		
Y507	8	13-8	114	STR		
Y608	8	4-0	48	8	3-5	
Y609	8	4-6	54	5	1-4 1/2	3-3
X501	20	7-1	148	7		
X502	32	7-11	264	STR		
		TOTAL	1599	LBS. X 2 Bridges = 3098 Lbs.		

For Bar Bending Diagram See Sheet 435/522

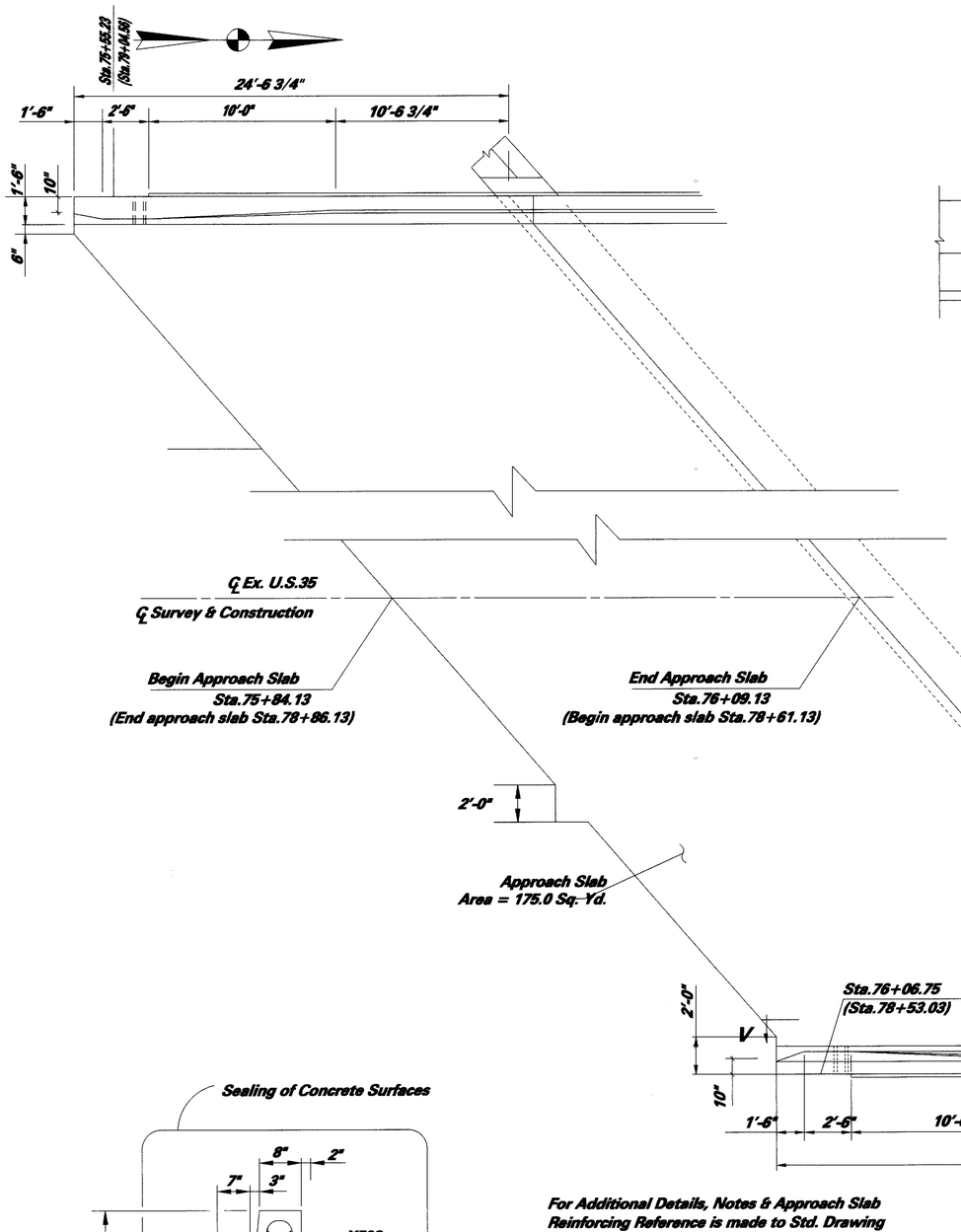
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1123 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

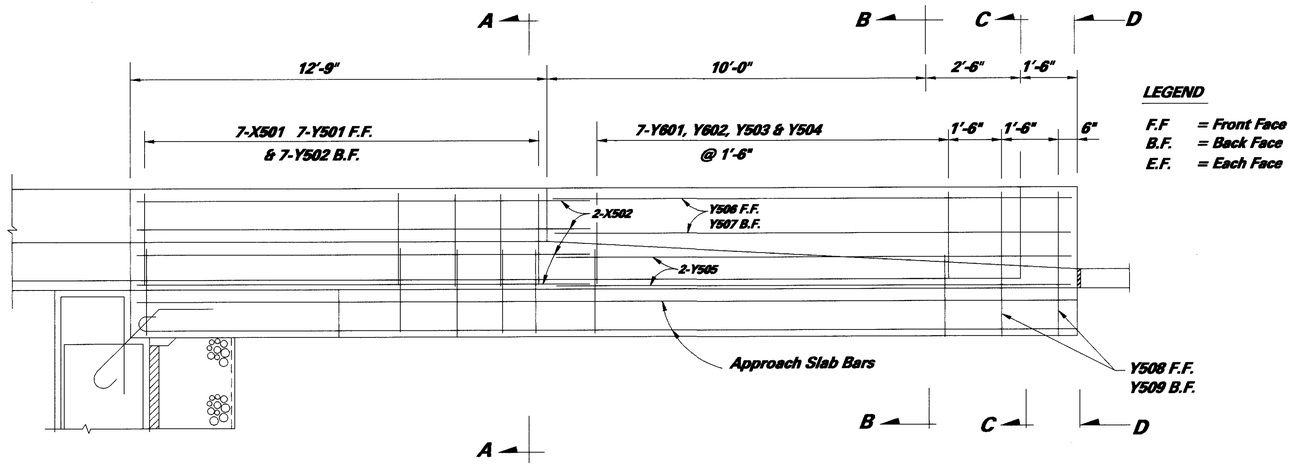
DESIGNED RT
DRAWN RT
CHECKED VK
REVIEWED
DATE
REVISED
STRUCTURE FILE NO.3400359

APPROACH SLAB DETAILS
BRIDGE NO. FAY-35-0366
U.S.35 OVER I-71

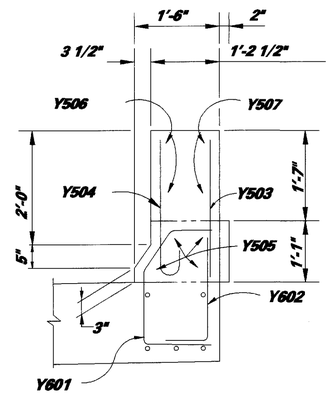
FAY-35-2.57

261
522

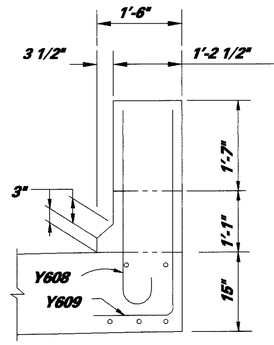




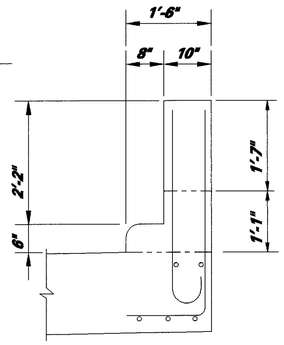
VIEW V-V



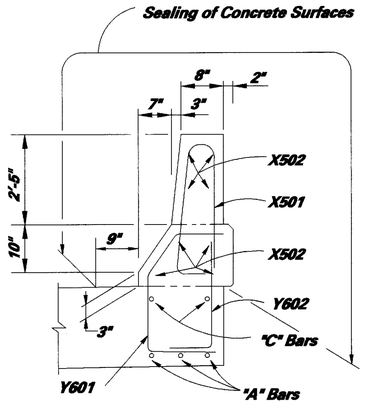
SECTION B-B



SECTION C-C



SECTION D-D



SECTION A-A

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

Stations in ( ) are for Forward Approach Slab.

The portion of the approach slab which extends under the parapet shall be included in the cost of Item 611.

All Parapet Concrete including transition is HPC.

REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	56	3-6	294	6	1-1 1/2	
Y602	56	2-9	231	5	0-10 1/2	2-0
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	9		
Y507	8	13-8	114	STR		
Y608	8	4-0	48	8	3-5	
Y609	8	5-0	54	5	1-8 1/2	3-5
X501	28	7-1	207	7		
X502	32	14-6	484	STR		
		TOTAL	1879	LBS.		

For Bar Bending Diagram See Sheet 452 522

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

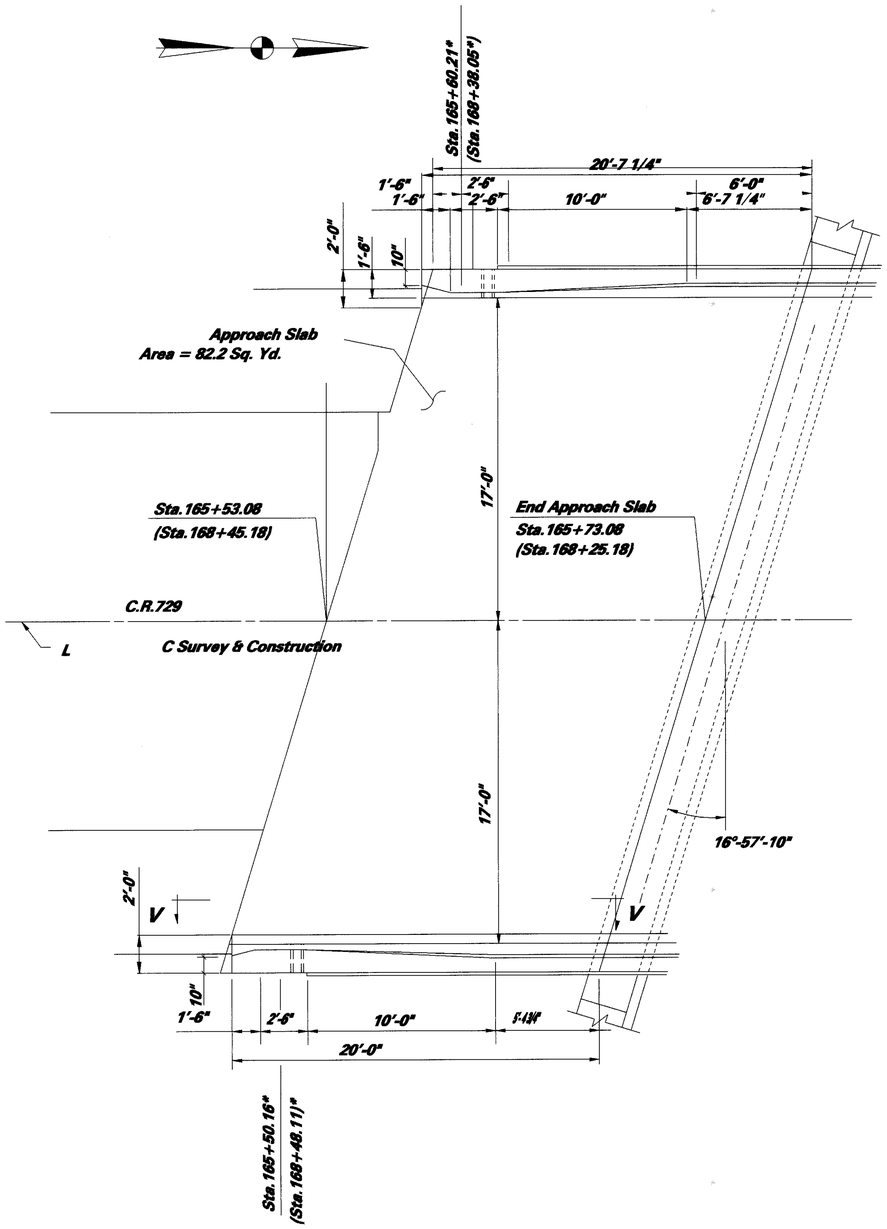
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-89
CHECKED	REVISED		
VK			

STRUCTURE FILE NO.2400308

APPROACH SLAB DETAILS

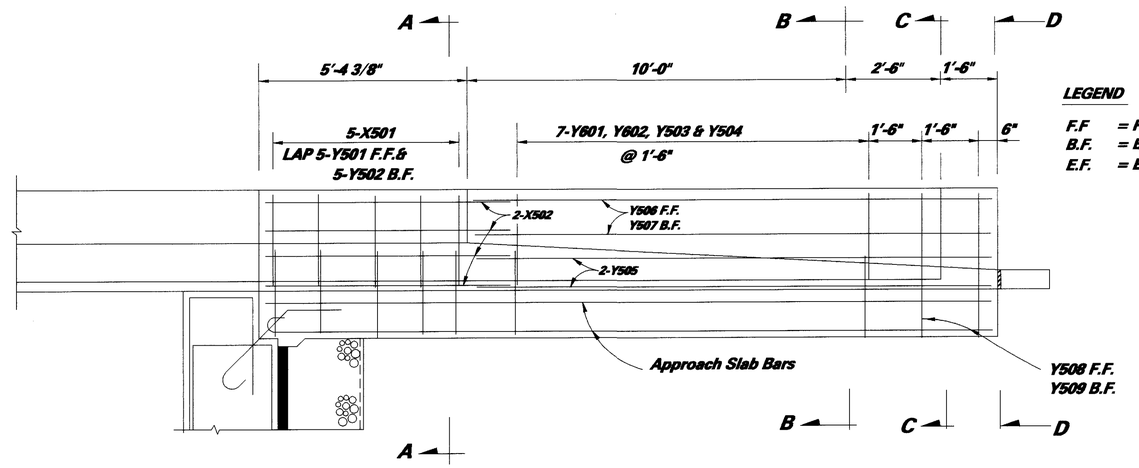
BRIDGE NO. FAY-35-0495
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57



PLAN - REAR APPROACH SLAB

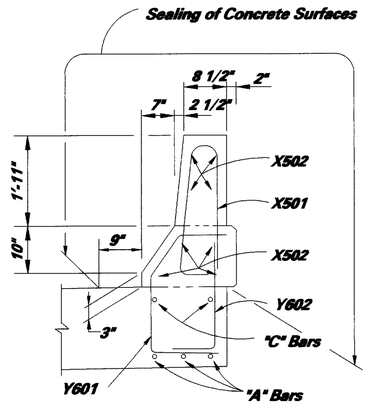
Foward Approach Slab Similar



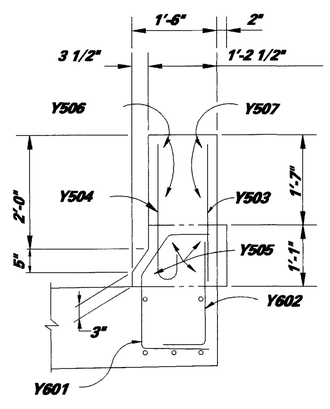
LEGEND

F.F = Front Face
B.F. = Back Face
E.F. = Each Face

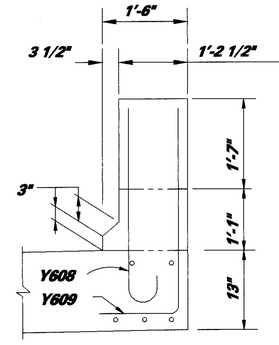
VIEW V-V



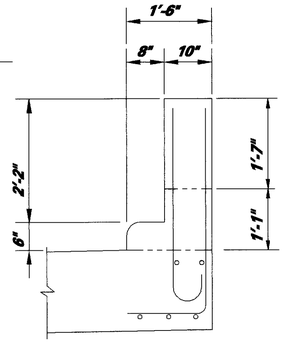
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

Stations in ( ) are for Forward Approach Slab.

The portion of the approach slab which extends under the parapet shall be included in the cost of Item 611.

All Parapet Concrete including Transition is HPC.

REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	48	3-5	246	6	1-0	
Y602	48	2-6	180	5	0-10 1/2	1-9
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	9		
Y507	8	13-8	114	STR		
Y608	8	3-10	46	8	3-3	
Y609	8	4-6	54	5	1-4 1/2	3-3
X501	20	6-0	125	7		
X502	32	8-6	283	STR		
		TOTAL	1368	LBS.		

For Bar Bending Diagram See Sheet 466/522

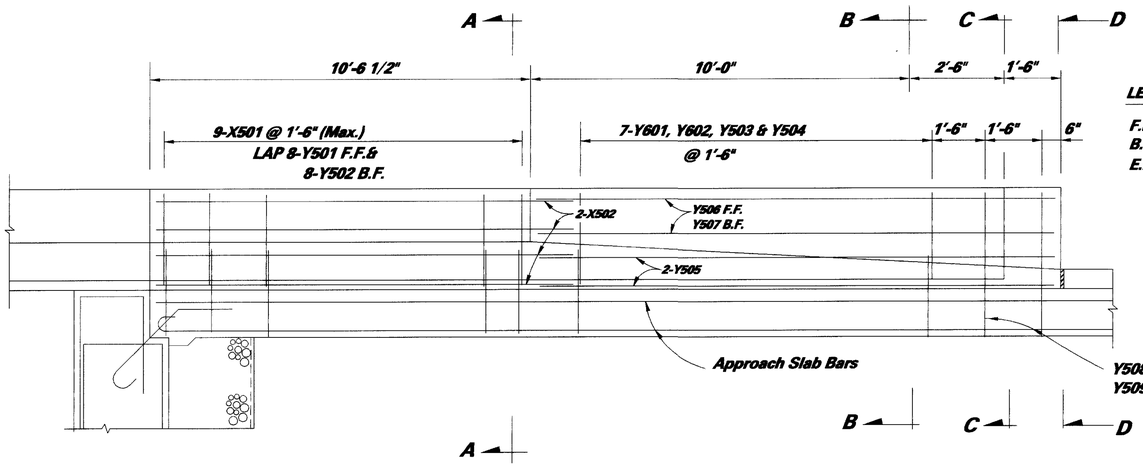
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1123 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT		
CHECKED	REVISED		
VK			

STRUCTURE FILE NO.

APPROACH SLAB DETAILS
BRIDGE NO. FAY-35-0518
SR 729 OVER US 35

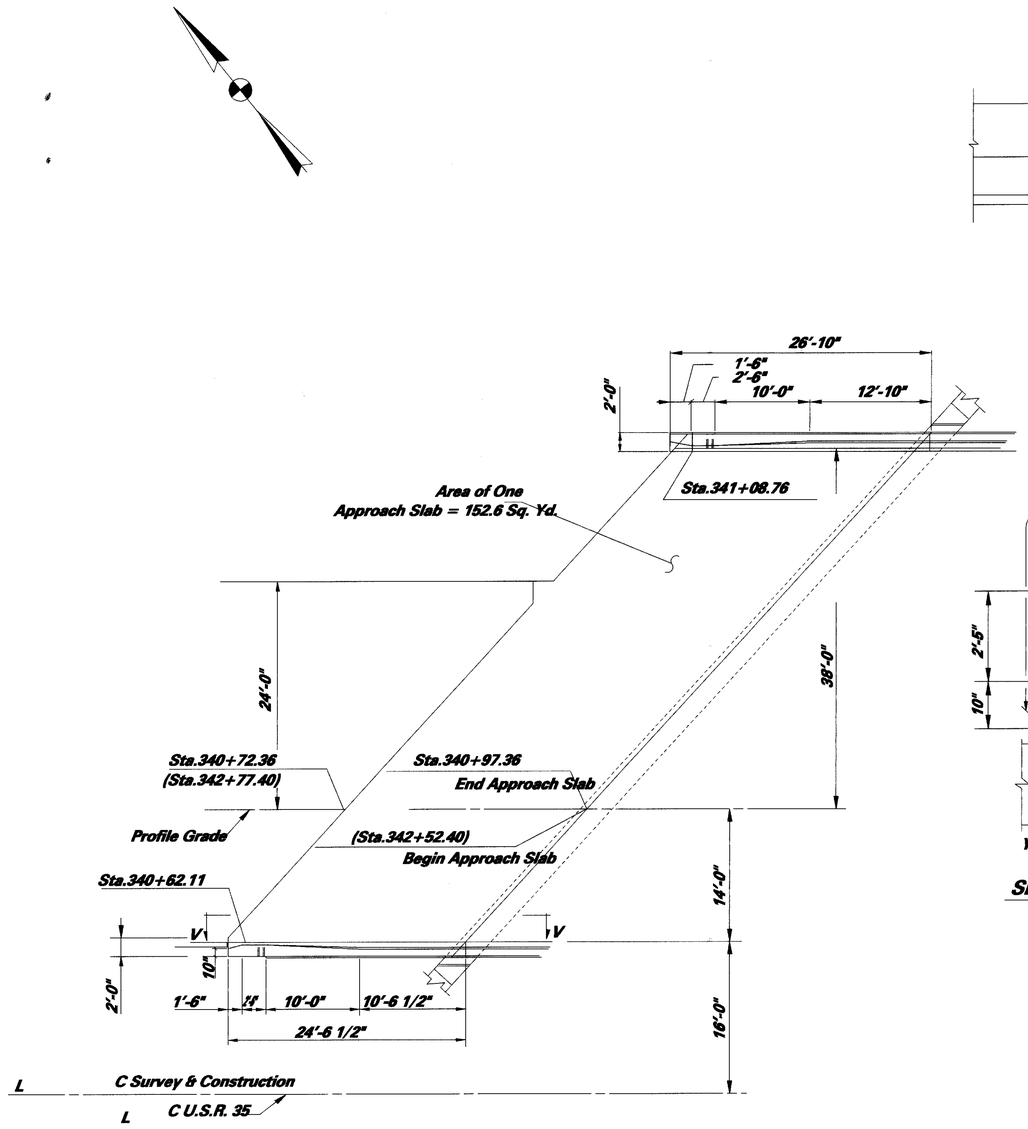
FAY-35-2.57



LEGEND

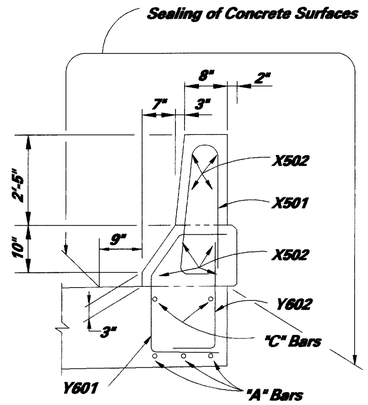
F.F = Front Face
B.F. = Back Face
E.F. = Each Face

VIEW V-V

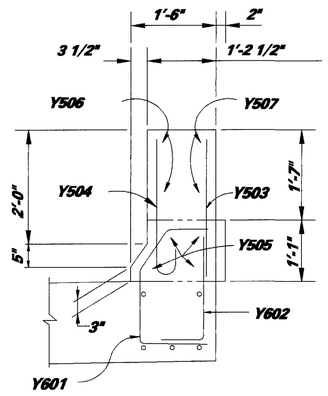


Rear Approach Slab - Left Bridge

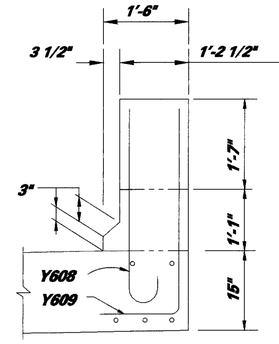
Similar At 3 Other Locations



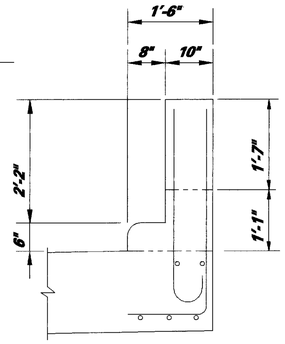
SECTION A-A



SECTION B-B



SECTION C-C



SECTION D-D

For Additional Details, Notes & Approach Slab Reinforcing Reference is made to Std. Drawing AS-1-81.

For Notes & Additional Details of the concrete parapet reference is made to Std. Drawing BR-1 & Std. Construction Drawing GR-3.1.

Stations in ( ) are for Forward Approach Slab.

The portion of the approach slab which extends under the parapet shall be included in the cost of Item 611.

All Parapet Concrete including transition is HPC.

REINFORCING STEEL LIST (ONE BRIDGE)

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B
Y601	60	3-6	315	5	1-1 1/2	
Y602	60	2-9	247	4	0-10 1/2	2-0
Y503	28	2-4	68	STR		
Y504	28	2-11	85	8	2-4	
Y505	16	13-8	228	STR		
Y506	8	13-8	114	7		
Y507	8	13-8	114	STR		
Y608	8	4-0	48	8	3-5	
Y609	8	5-0	60	4	1-8 1/2	3-5
X501	36	7-1	266	6		
X502	32	14-10	495	STR		
		TOTAL	1767	LBS. X 2 Bridges = 3534 Lbs.		

For Bar Bending Diagram See Sheet 482 522

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED CEE | DATE 2-24-99 | STRUCTURE FILE NO.3400448

DRAWN RT | REVISED

DESIGNED RT | CHECKED VK

APPROACH SLAB DETAILS

BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

HORIZONTAL SCALE IN FEET
0 30 60 120

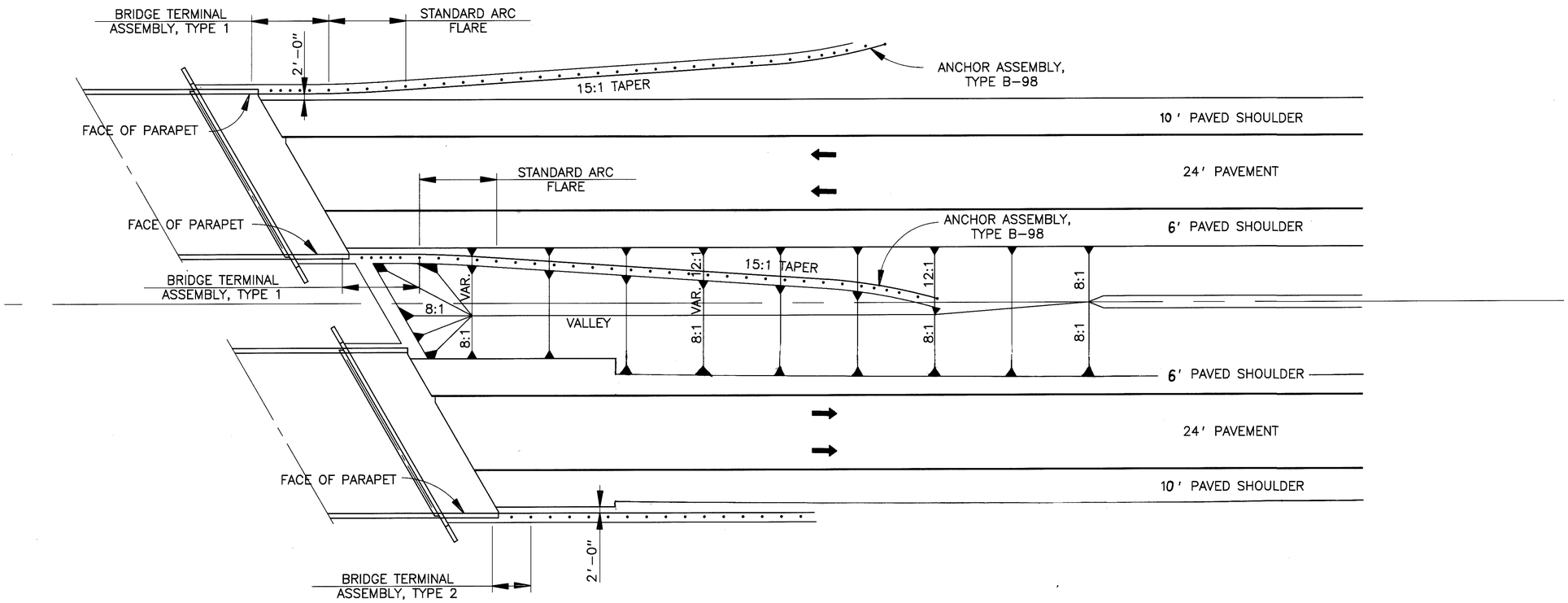
CALCULATED

CHECKED

GRADING PLAN

FAY-35-2.57

265
522



Note: 12' Shoulder (Paved) width is to remain unchanged, off the Approach Slab, until 5ft past the Sodded Flume.

0 10 20 40 HORIZONTAL SCALE IN FEET

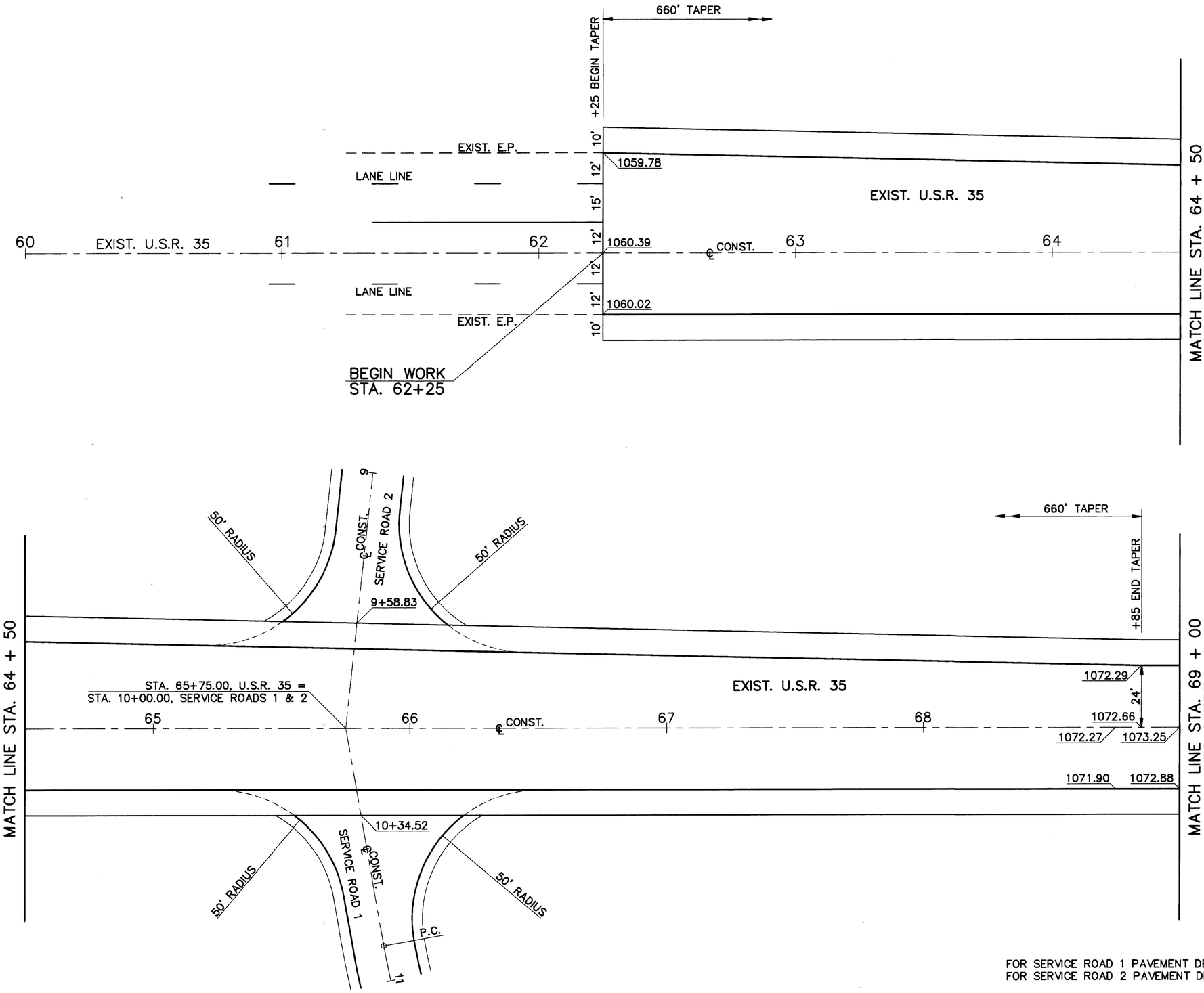
CALCULATED | CHECKED

GUARDRAIL AT BRIDGES

FAY-35-2.57

REVISED 10-99
BY FYC

266
522



FOR SERVICE ROAD 1 PAVEMENT DETAILS, SEE SHEET 275
FOR SERVICE ROAD 2 PAVEMENT DETAILS, SEE SHEET 275

HORIZONTAL SCALE IN FEET
0 10 20 40

CALCULATED J.L.G.
CHECKED W.G.Y.

PAVEMENT DETAILS EXIST. U.S.R. 35
STA. 60 + 00 TO STA. 69 + 00

FAY-35-2.57

MATCH LINE STA. 70+50

EXIST. U.S.R. 35

12' RT. STA. 74 + 57.81 EX. U.S.R. 35 =
P.C. STA. 229 + 01.04 RAMP CC

1078.54 1079.31 1080.04 1080.75 1081.42 1082.05 +97.56 1082.66 1083.23 1083.77 1084.28 1084.75 1085.19 1085.60 1085.97 1086.32 1086.63 1086.90 1087.15 1087.36 1087.54 1087.69 1087.80

71 72 73 74 75 76

1078.35 1079.12 1079.85 1080.56 1081.23 1081.86 1082.47 1083.04 1083.58 1084.09 1084.56 1085.00 1085.41 1085.78 1086.13 1086.44 1086.71 1086.96 1087.17 1087.35 1087.50 1087.61

10' 24' 12' 10'

MATCH LINE STA. 226+00

1077.69 1078.37 1078.99 1079.57 1080.09 1080.57 1079.68 1080.27 1080.79 1081.27 1081.69 1082.07 1082.39 1082.67 1082.89 1083.07 1083.19 1083.27 1083.29 1082.57 1082.59 1082.49 1082.37 1082.19 1081.97 1081.69 1081.37 1080.99

227 228 229 230

6' 10' 16' 10' 3'

RAMP C

+48.46

EXIST. USR35 PAVEMENT → RAMP C PAVEMENT

229+13.33

+84.06

18'

1086.30 1085.95 1085.45 1084.60 1083.83 1083.22 1082.78 1082.55 1082.30

1085.06 1084.25 1083.59 1083.08 1082.72 1082.45

1083.27 1083.19 1082.49 1082.37 1082.78 1082.19 1081.97

RAMP CC

230+10.32

P.T. STA. 230 + 41.01 RAMP CC

STA. 230 + 59.33 RAMP CC =
20' L.T. STA. 230 + 59.33 RAMP C
BEGIN TAPER

660' TAPER @ 30:1

230+10.04

P.T. STA. 230 + 59.33 RAMP C

MATCH LINE STA. 231+00
SEE SHEET

SEE SHEET
MATCH LINE STA. 69+00

MATCH LINE STA. 70+50

EXIST. U.S.R. 35

1073.25 1074.21 1075.14 1076.04 1076.91 1077.74

70

1073.06 1074.02 1074.95 1078.85 1076.72 1077.55

1072.87 1073.83 1074.68 1078.48 1076.19 1076.97

12' 12' 10'

RAMP C

MATCH LINE STA. 226+00

P.C. STA. 225 + 40.19 RAMP C =
24' RT. STA. 69 + 87.48 EX. U.S.R. 35

CURVE DATA RAMP C
P.I. STA. 228+06.35
P.C. STA. 225+40.19
P.T. STA. 230+59.33
Δ = 31°-08'-53.5" RT.
Dc = 6°-00'-00"
R = 954.93'
T = 266.16'
L = 519.14'
E = 36.40'
SE = .044 : 1

CURVE DATA RAMP CC
P.I. STA. 229+94.24
P.C. STA. 229+01.04
P.T. STA. 230+41.01
Δ = 96°-37'-33.2" LT.
Dc = 69°-01'-52"
R = 83.00'
T = 93.20'
L = 139.97'
E = 41.80'

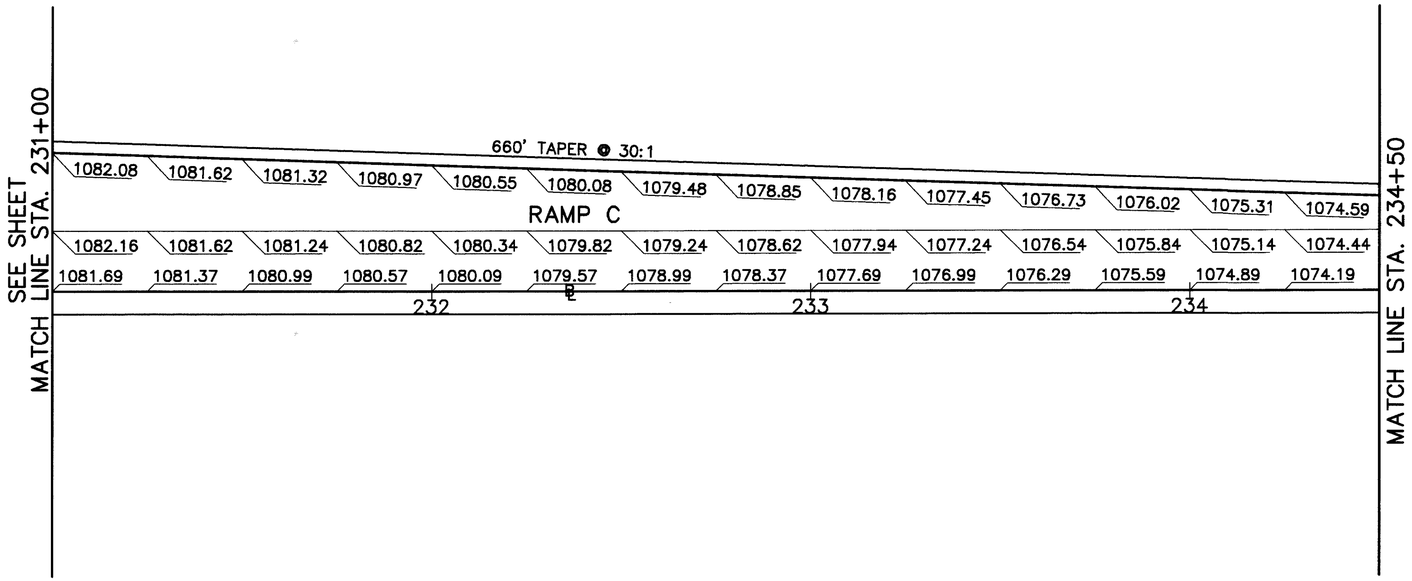
HORIZONTAL SCALE IN FEET
0 10 20 40

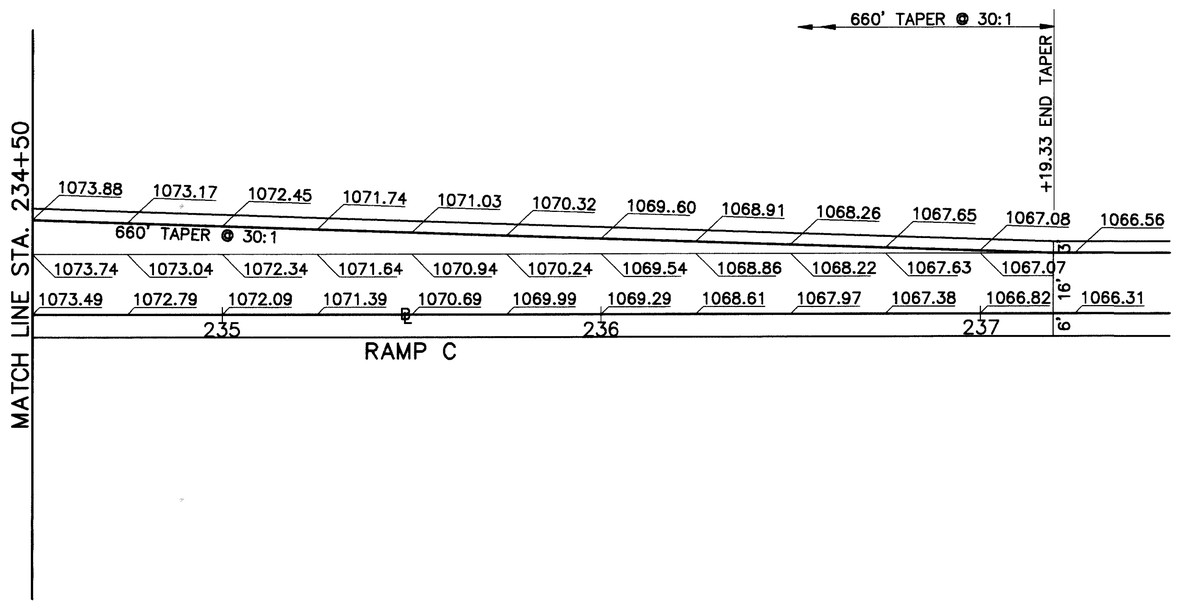
CALCULATED J.L.G.
CHECKED W.G.Y.

PAVEMENT DETAILS EXIST. U.S.R. 35 & RAMP C
STA. 69 + 00 TO STA. 76 + 00

FAY-35-2.57

268
522





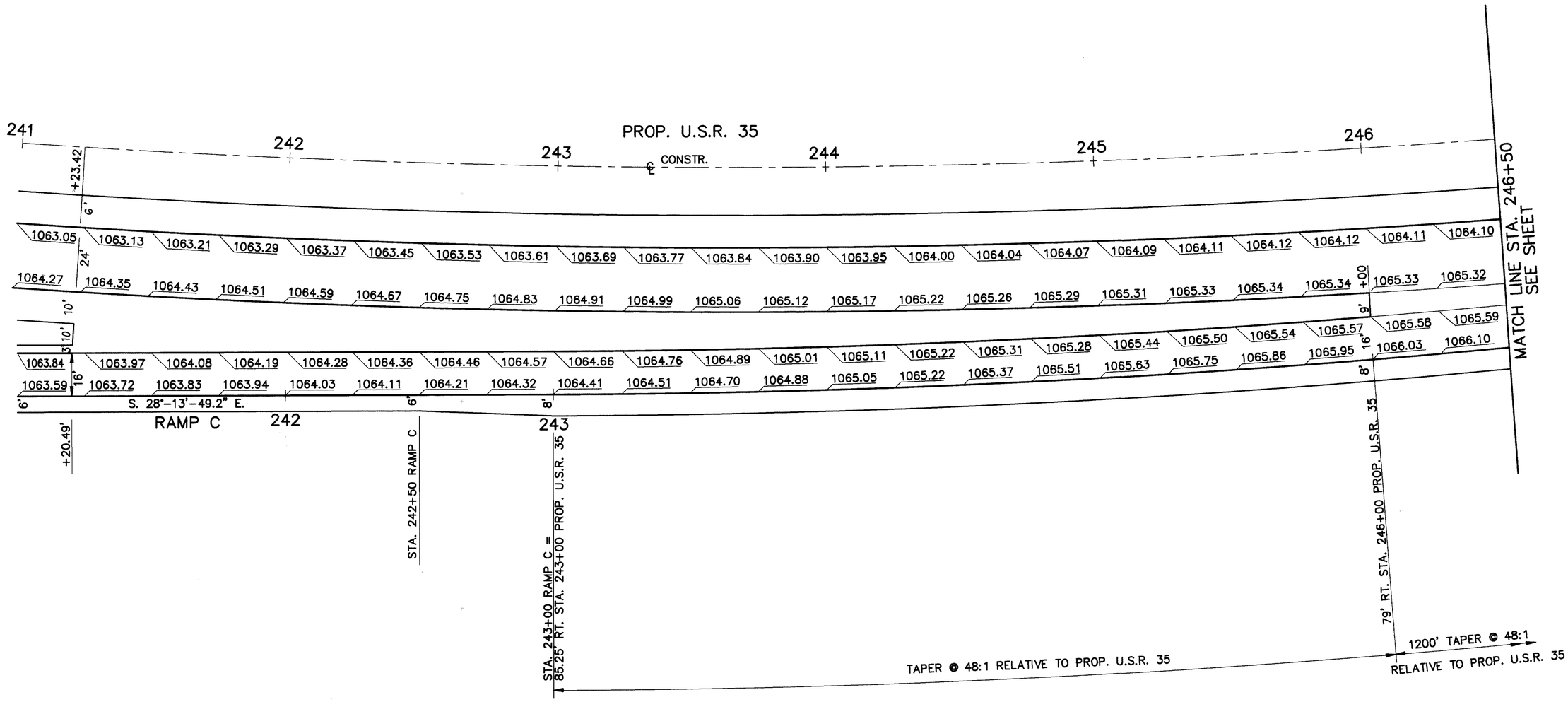
CALCULATED J.L.G.
CHECKED W.G.Y.

0 10 20 40
HORIZONTAL SCALE IN FEET

PAVEMENT DETAILS RAMP C
STA. 231 + 00 TO STA. 237 + 50

FAY-35-2.57

269
522



0 10 20 40
HORIZONTAL SCALE IN FEET

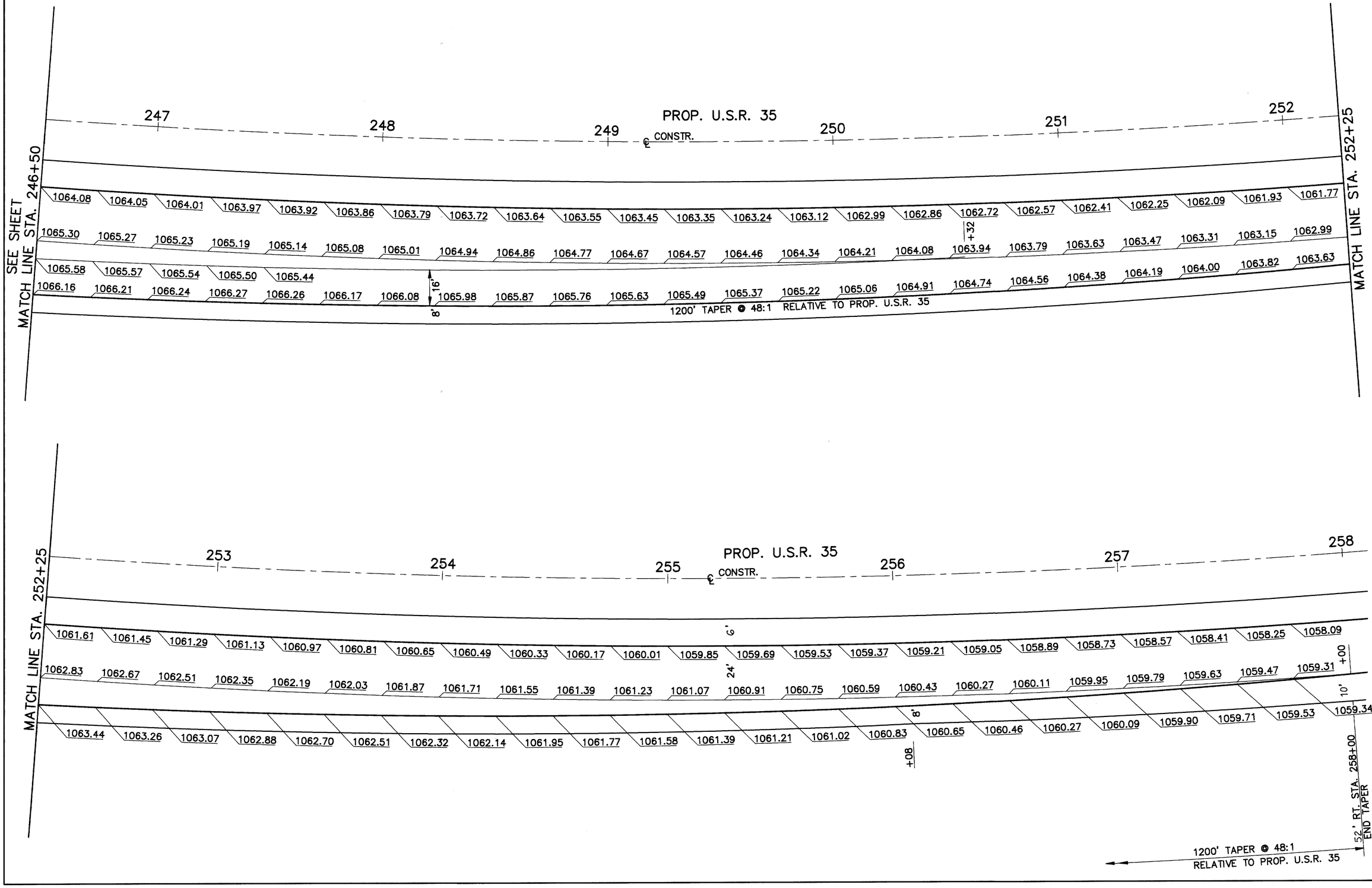
CALCULATED J.L.G.
CHECKED W.G.Y.

REVISED 10-99
BY F Y C

PAVEMENT DETAILS RAMP C
STA. 241 + 00 TO STA. 246 + 50

FAY-35-2.57

270
522



0 10 20 40
HORIZONTAL SCALE IN FEET

CALCULATED J.L.G.
CHECKED W.G.Y.

REVISED 10-99
BY F Y C

PAVEMENT DETAILS RAMP C
STA. 246 + 50 TO STA. 258 + 00

FAY-35-2.57

271
522

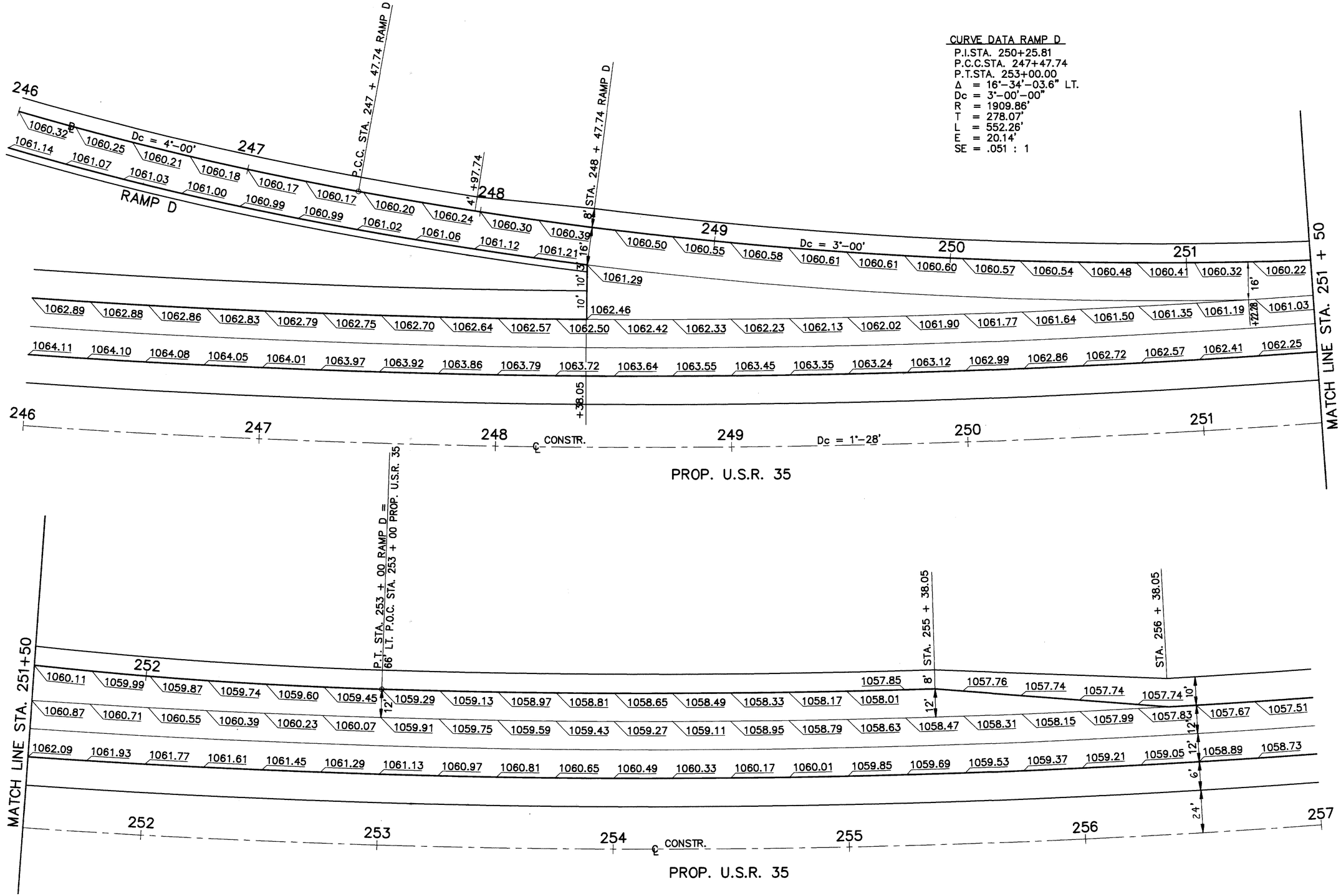
CALCULATED J.L.G.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 10 20 40

PAVEMENT DETAILS RAMP D
STA. 236 + 98.70 TO 241 + 25

FAY-35-2.57

272
522

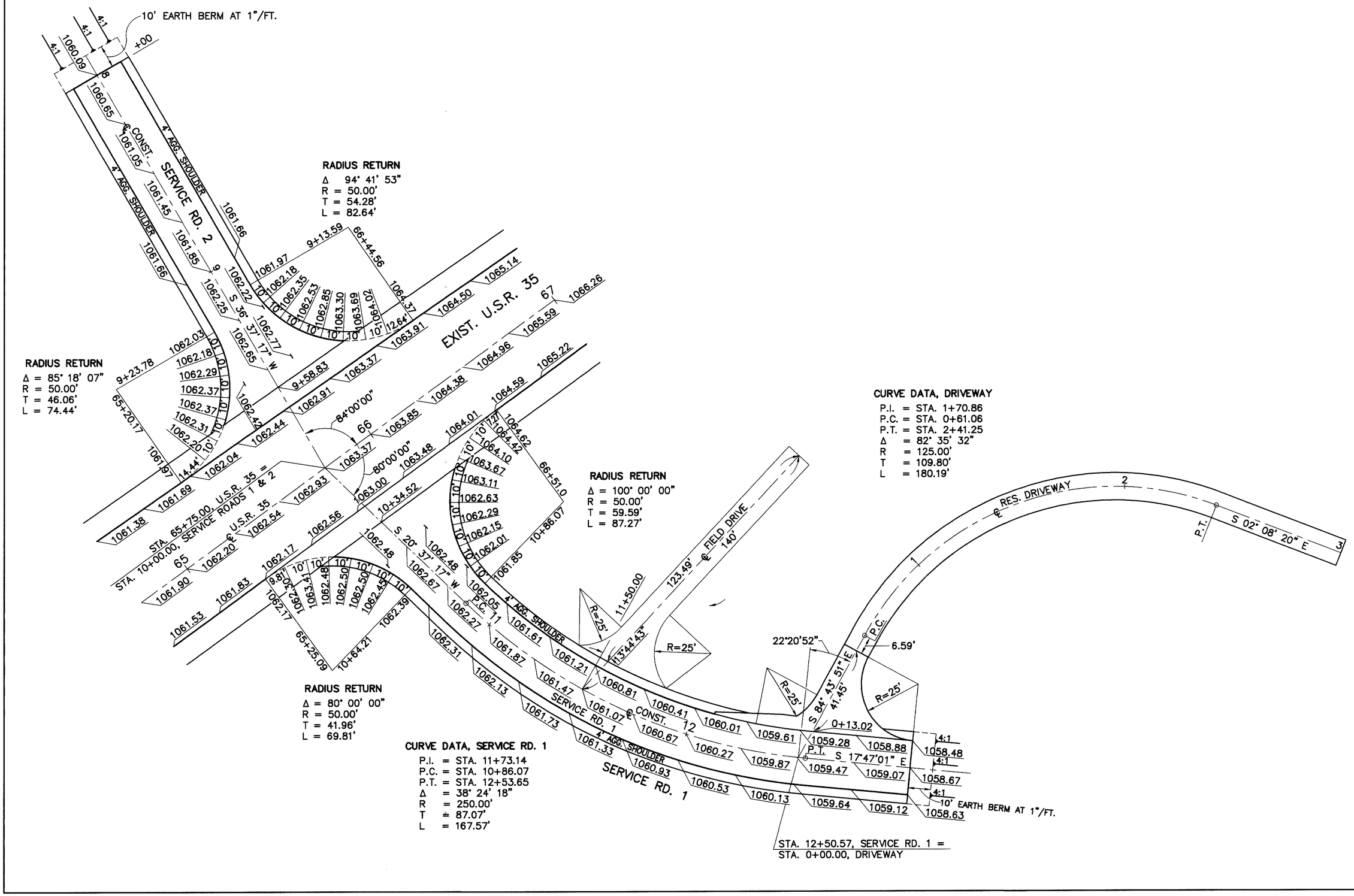


REVISED 10-99 BY FYC
CALCULATED J.L.G. CHECKED W.G.Y.
HORIZONTAL SCALE IN FEET 0 10 20 40

PAVEMENT DETAILS RAMP D
STA. 246 + 00 TO STA. 257 + 00

FAY-35-2.57

273
522



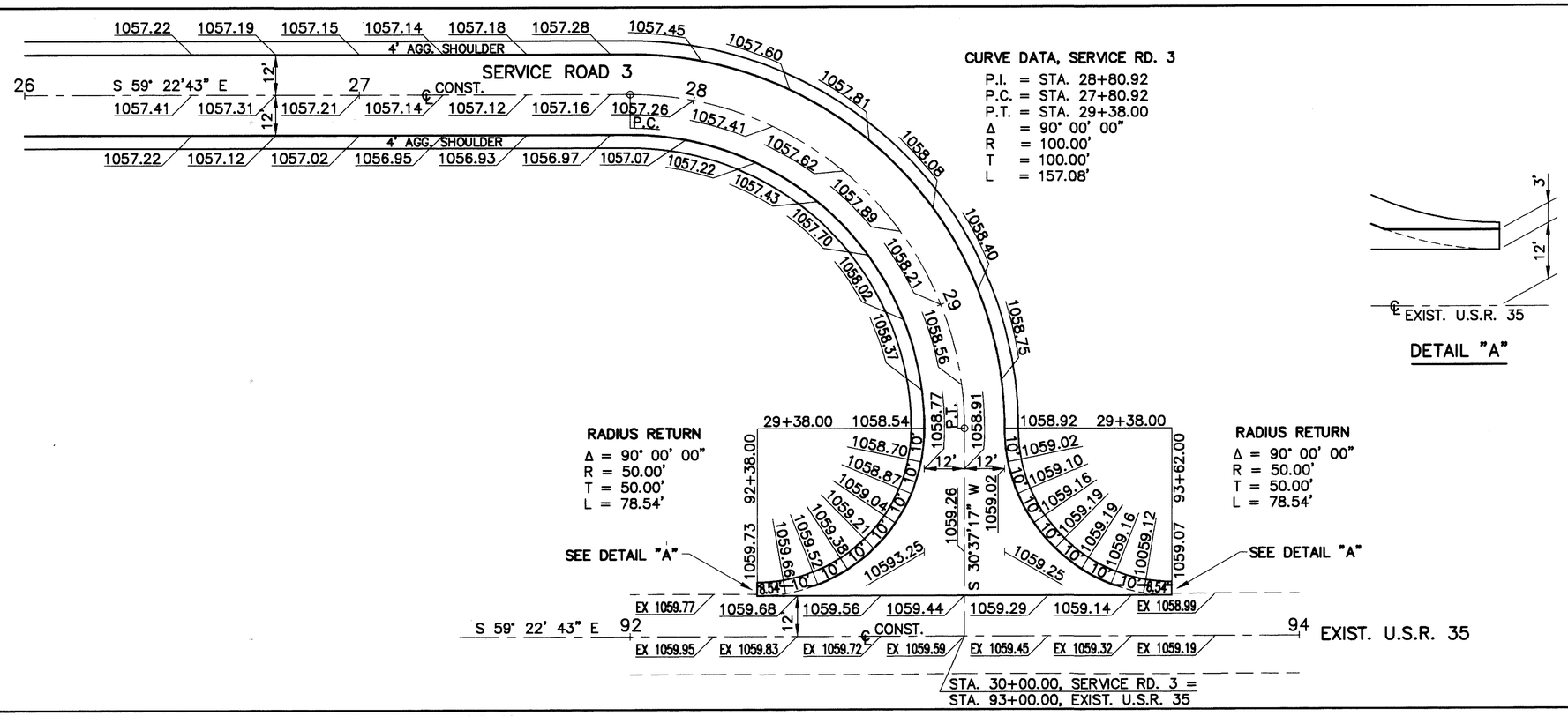
0 10 20 40 HORIZONTAL SCALE IN FEET

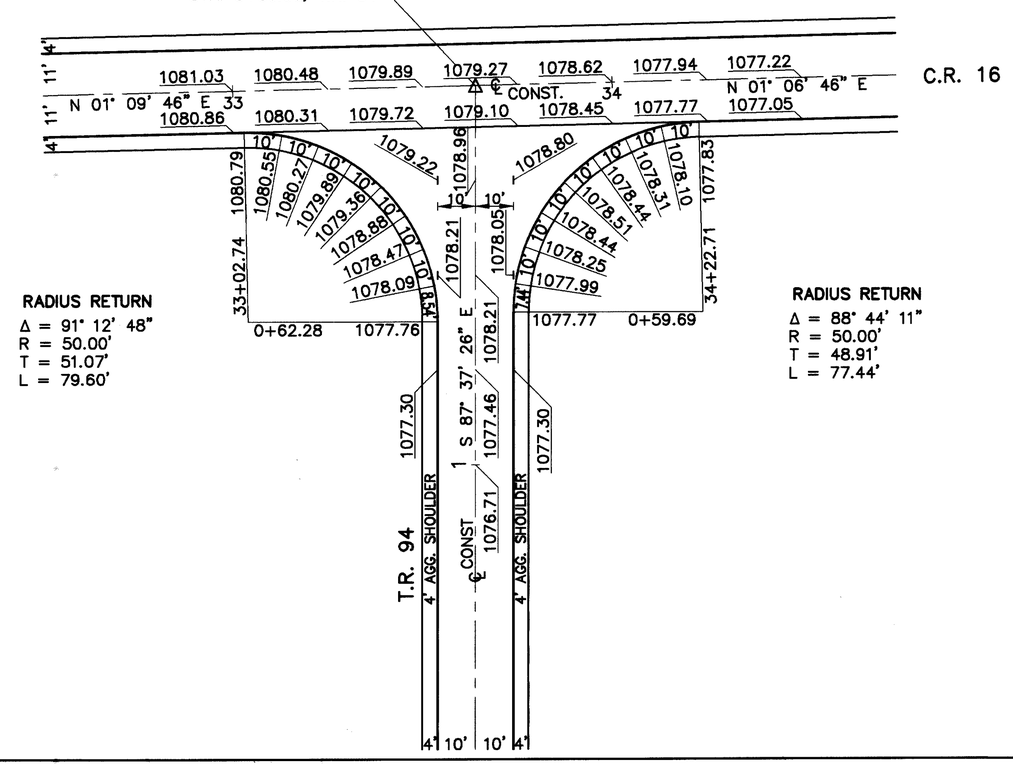
CALCULATED J.L.G. CHECKED W.G.Y.

PAVEMENT DETAILS SERVICE ROADS STA. 65+75

FAY-35-2.57

274
522





CALCULATED J.L.G. CHECKED W.G.Y.

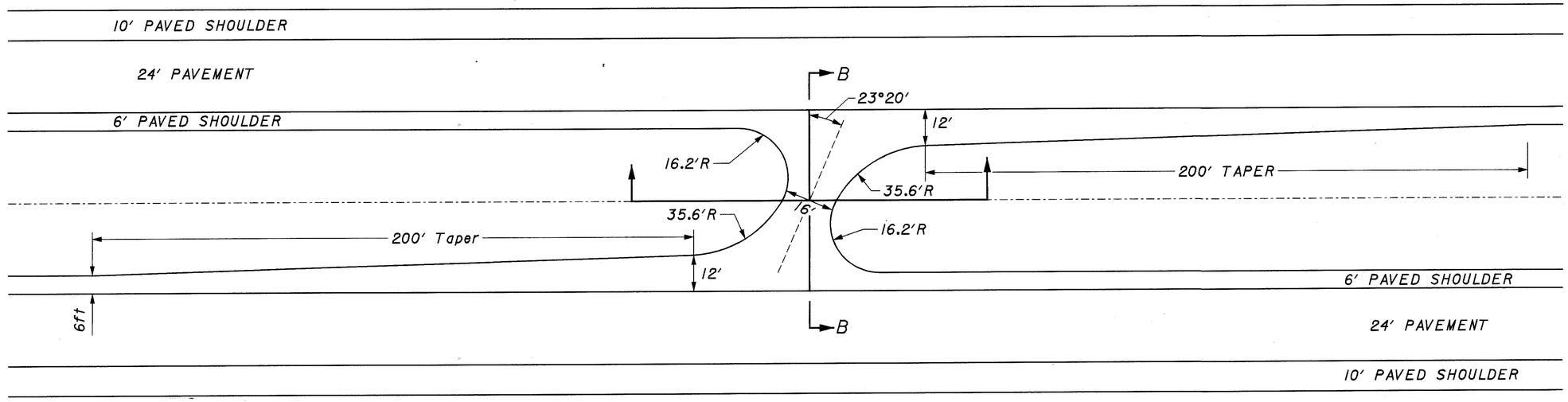
HORIZONTAL SCALE IN FEET 0 10 20 40

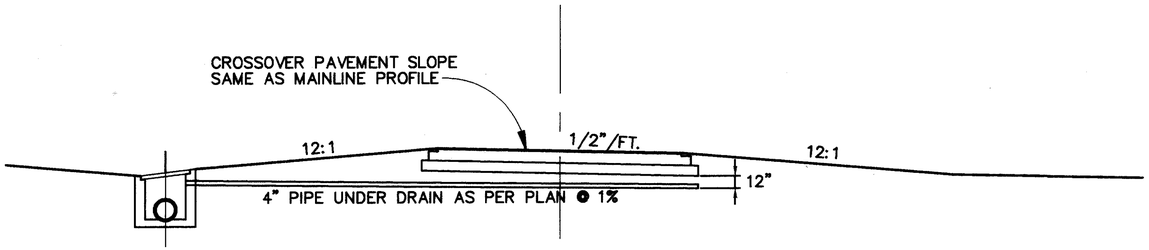
PAVEMENT DETAILS
EXIST. U.S.R. 35 & SERVICE ROAD 3

C.R. 16 & T.R. 94

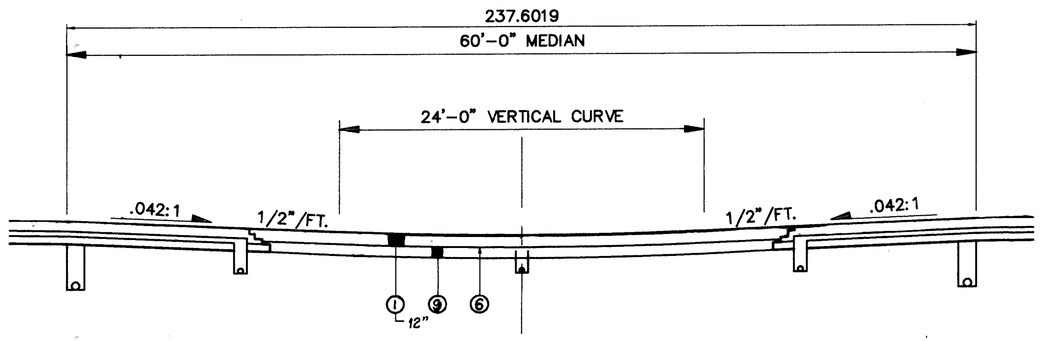
FAY-35-2.57

275
522



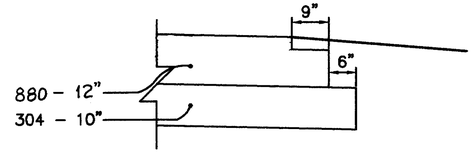


SECTION A–A



Note: Use legend shown on Sheet 5.

SECTION B–B



EDGE COURSE DETAIL

CROSSOVER LOCATIONS
STA.280+28 MAINLINE
STA.382+53 MAINLINE

CALCULATED	CHECKED
J.L.G.	W.G.Y.

REVISED	BY
12/99	FYC

STANDARD U–TURN MEDIAN OPENING

FAY–35–2.57

SUPERELEVATION TABLES

PROPOSED U.S.R. 35

CURVE No. 1

SE = 0.044 Dc = 01° 15' RT.

OUTSIDE EDGE PAVED SHOULDER	EDGE PAVEMENT	CENTER	P. G. EDGE PAVEMENT	MEDIAN EDGE PAVED SHOULDER	STATION	MEDIAN EDGE PAVED SHOULDER	P. G. EDGE PAVEMENT	CENTER	EDGE PAVEMENT	OUTSIDE EDGE PAVED SHOULDER
1063.47	1063.89	1064.08	1063.89	1063.64	172+00	1063.64	1063.89	1064.08	1063.89	1063.47
1063.63	1064.05	1064.14	1063.95	1063.70	172+25	1063.70	1063.95	1064.04	1063.85	1063.43
1063.78	1064.20	1064.20	1064.01	1063.76	172+48	1063.76	1064.01	1064.01	1063.82	1063.40
1063.79	1064.21	1064.20	1064.01	1063.76	172+50	1063.76	1064.01	1064.00	1063.81	1063.39
1063.95	1064.37	1064.26	1064.07	1063.82	172+75	1063.82	1064.07	1063.96	1063.77	1063.35
1064.08	1064.50	1064.31	1064.12	1063.87	172+96	1063.87	1064.12	1063.93	1063.74	1063.32
1064.11	1064.53	1064.33	1064.13	1063.88	173+00	1063.88	1064.13	1063.93	1063.73	1063.31
1064.27	1064.69	1064.44	1064.19	1063.94	173+25	1063.94	1064.19	1063.94	1063.69	1063.27
1064.43	1064.85	1064.55	1064.25	1064.00	173+50	1064.00	1064.25	1063.95	1063.65	1063.23
1064.60	1065.01	1064.66	1064.31	1064.06	173+75	1064.06	1064.31	1063.96	1063.61	1063.19
1064.60	1065.01	1064.66	1064.31	1064.06	PC 173+75.49	1064.06	1064.31	1063.96	1063.61	1063.19
1064.81	1065.18	1064.78	1064.38	1064.13	174+00	1064.16	1064.38	1063.98	1063.58	1063.16
1065.04	1065.36	1064.91	1064.46	1064.21	174+25	1064.26	1064.46	1064.01	1063.56	1063.14
1065.27	1065.55	1065.05	1064.55	1064.30	174+50	1064.38	1064.55	1064.05	1063.55	1063.13
1065.41	1065.67	1065.14	1064.61	1064.34	174+64	1064.46	1064.61	1064.08	1063.55	1063.11
				FULL SUPER						
PG +0.75	PG +1.06	PG +0.53		PG -0.53		PG -0.31		PG -0.53	PG -1.06	PG -1.59
PG +0.75	PG +1.06	PG +0.53		PG -0.53		PG -0.31		PG -0.53	PG -1.06	PG -1.59
				FULL SUPER						
1068.81	1069.07	1068.54	1068.01	1067.74	215+50	1067.86	1068.01	1067.48	1066.95	1066.51
1068.59	1068.89	1068.41	1067.93	1067.68	215+75	1067.75	1067.93	1067.45	1066.97	1066.55
1068.37	1068.71	1068.28	1067.85	1067.60	216+00	1067.64	1067.85	1067.42	1066.99	1066.57
1068.15	1068.53	1068.15	1067.77	1067.52	216+25	1067.54	1067.77	1067.39	1067.01	1066.59
1068.02	1068.43	1068.08	1067.73	1067.48	PT 216+38.09	1067.48	1067.73	1067.38	1067.03	1066.61
1067.93	1068.35	1068.02	1067.69	1067.44	216+50	1067.44	1067.69	1067.36	1067.03	1066.61
1067.75	1068.17	1067.89	1067.61	1067.36	216+75	1067.36	1067.61	1067.33	1067.05	1066.63
1067.57	1067.99	1067.76	1067.53	1067.28	217+00	1067.28	1067.53	1067.30	1067.07	1066.65
1067.43	1067.85	1067.66	1067.47	1067.22	217+18	1067.22	1067.47	1067.28	1067.09	1066.67
1067.38	1067.80	1067.64	1067.45	1067.20	217+25	1067.20	1067.45	1067.29	1067.10	1066.68
1067.20	1067.62	1067.56	1067.37	1067.12	217+50	1067.12	1067.37	1067.31	1067.12	1066.70
1067.09	1067.51	1067.51	1067.32	1067.07	217+66	1067.07	1067.32	1067.32	1067.13	1066.71
1067.02	1067.44	1067.48	1067.29	1067.04	217+75	1067.04	1067.29	1067.33	1067.14	1066.72
1066.84	1067.26	1067.40	1067.21	1066.96	218+00	1066.96	1067.21	1067.35	1067.16	1066.74
1066.75	1067.17	1067.36	1067.17	1066.92	218+14	1066.92	1067.17	1067.36	1067.17	1066.75

CURVE No. 2

SE = 0.051 Dc = 01° 28' LT.

OUTSIDE EDGE PAVED SHOULDER	EDGE PAVEMENT	CENTER	P. G. EDGE PAVEMENT	MEDIAN EDGE PAVED SHOULDER	STATION	MEDIAN EDGE PAVED SHOULDER	P. G. EDGE PAVEMENT	CENTER	EDGE PAVEMENT	OUTSIDE EDGE PAVED SHOULDER
1063.75	1064.17	1064.36	1064.17	1063.92	227+50	1063.92	1064.17	1064.36	1064.17	1063.75
1063.57	1063.99	1064.18	1064.09	1063.84	227+75	1063.84	1064.09	1064.28	1064.19	1063.77
1063.41	1063.83	1064.02	1064.02	1063.77	227+98	1063.77	1064.02	1064.21	1064.21	1063.79
1063.39	1063.81	1064.00	1064.01	1063.76	228+00	1063.76	1064.01	1064.20	1064.21	1063.79
1063.21	1063.63	1063.82	1063.93	1063.68	228+25	1063.68	1063.93	1064.12	1064.23	1063.81
1063.06	1063.48	1063.67	1063.86	1063.61	228+46	1063.61	1063.86	1064.05	1064.24	1063.82
1063.03	1063.45	1063.65	1063.85	1063.60	228+50	1063.60	1063.85	1064.05	1064.25	1063.83
1062.85	1063.27	1063.52	1063.77	1063.52	228+75	1063.52	1063.77	1064.02	1064.27	1063.85
1062.67	1063.09	1063.39	1063.69	1063.44	229+00	1063.44	1063.69	1063.99	1064.29	1063.87
1062.49	1062.91	1063.26	1063.61	1063.36	229+25	1063.36	1063.61	1063.96	1064.31	1063.90
1062.35	1062.77	1063.16	1063.55	1063.32	PC 229+44.26	1063.30	1063.55	1063.94	1064.33	1063.96
1062.31	1062.73	1063.13	1063.53	1063.31	229+50	1063.28	1063.53	1063.93	1064.33	1063.96
1062.13	1062.55	1063.00	1063.45	1063.26	229+75	1063.20	1063.45	1063.90	1064.35	1064.02
1061.95	1062.37	1062.87	1063.37	1063.20	230+00	1063.12	1063.37	1063.87	1064.37	1064.09
1061.73	1062.19	1062.74	1063.29	1063.14	230+25	1063.02	1063.29	1063.84	1064.39	1064.15
1061.51	1062.01	1062.61	1063.21	1063.09	230+50	1062.91	1063.21	1063.81	1064.41	1064.21
1061.46	1061.97	1062.58	1063.19	1063.08	230+56	1062.88	1063.19	1063.80	1064.41	1064.22
				FULL SUPER						
PG -1.83	PG -1.22	PG -0.61		PG -0.23		PG -0.61		PG +0.61	PG +1.22	PG +0.99
PG -1.83	PG -1.22	PG -0.61		PG -0.23		PG -0.61		PG +0.61	PG +1.22	PG +0.99
				FULL SUPER						
1049.36	1049.87	1050.48	1051.09	1050.98	269+25	1050.78	1051.09	1051.70	1052.31	1052.12
1049.39	1049.86	1050.42	1050.98	1050.84	269+50	1050.70	1050.98	1051.54	1052.10	1051.87
1049.42	1049.84	1050.35	1050.86	1050.70	269+75	1050.60	1050.86	1051.37	1051.88	1051.60
1049.41	1049.83	1050.29	1050.75	1050.56	270+00	1050.50	1050.75	1051.21	1051.67	1051.35
1049.40	1049.82	1050.23	1050.64	1050.42	270+25	1050.39	1050.64	1051.05	1051.46	1051.10
1049.39	1049.81	1050.20	1050.59	1050.36	PT 270+37.24	1050.34	1050.59	1050.98	1051.37	1051.00
1049.40	1049.82	1050.18	1050.54	1050.30	270+50	1050.29	1050.54	1050.90	1051.26	1050.86
1049.39	1049.81	1050.12	1050.43	1050.18	270+75	1050.18	1050.43	1050.74	1051.05	1050.63
1049.39	1049.81	1050.07	1050.33	1050.08	271+00	1050.08	1050.33	1050.59	1050.85	1050.43
1049.39	1049.81	1050.02	1050.23	1049.98	271+25	1049.98	1050.23	1050.44	1050.65	1050.23
1049.39	1049.81	1050.00	1050.19	1049.94	271+35	1049.94	1050.19	1050.38	1050.57	1050.15
1049.40	1049.82	1050.01	1050.14	1049.89	271+50	1049.89	1050.14	1050.33	1050.46	1050.04
1049.40	1049.82	1050.01	1050.04	1049.79	271+75	1049.79	1050.04	1050.23	1050.26	1049.84
1049.40	1049.82	1050.01	1050.01	1049.76	271+83	1049.76	1050.01	1050.20	1050.20	1049.78
1049.41	1049.83	1050.02	1049.95	1049.70	272+00	1049.70	1049.95	1050.14	1050.07	1049.65
1049.42	1049.84	1050.03	1049.86	1049.61	272+25	1049.61	1049.86	1050.05	1049.88	1049.46
1049.42	1049.84	1050.03	1049.84	1049.59	272+31	1049.59	1049.84	1050.03	1049.84	1049.42

CALCULATED J.L.G. CHECKED W.G.Y.
REVISED 10-99 BY GFR

SUPERELEVATION TABLES - CURVE 1 & 2

FAY-35-2.57

SUPERELEVATION TABLES

PROPOSED U.S.R. 35

CURVE No. 3

SE = 0.036 Dc = 01° 00' RT.

OUTSIDE EDGE PAVED SHOULDER	EDGE PAVEMENT	CENTER	P. G. EDGE PAVEMENT	MEDIAN EDGE PAVED SHOULDER	STATION	MEDIAN EDGE PAVED SHOULDER	P. G. EDGE PAVEMENT	CENTER	EDGE PAVEMENT	OUTSIDE EDGE PAVED SHOULDER
1044.28	1044.70	1044.89	1044.70	1044.45	294+50	1044.45	1044.70	1044.89	1044.70	1044.28
1044.43	1044.85	1044.94	1044.75	1044.50	294+75	1044.50	1044.75	1044.84	1044.65	1044.23
1044.58	1045.00	1045.00	1044.81	1044.56	294+98	1044.56	1044.81	1044.81	1044.62	1044.20
1044.59	1045.01	1045.00	1044.81	1044.56	295+00	1044.56	1044.81	1044.80	1044.61	1044.19
1044.75	1045.17	1045.06	1044.87	1044.62	295+25	1044.62	1044.87	1044.76	1044.57	1044.15
1044.88	1045.30	1045.11	1044.92	1044.67	295+46	1044.67	1044.92	1044.73	1044.54	1044.12
1044.91	1045.33	1045.13	1044.93	1044.68	295+50	1044.68	1044.93	1044.73	1044.53	1044.11
1045.07	1045.49	1045.24	1044.99	1044.74	295+75	1044.74	1044.99	1044.74	1044.49	1044.07
1045.17	1045.59	1045.31	1045.03	1044.78	PC 295+90.26	1044.78	1045.03	1044.75	1044.47	1044.05
1045.23	1045.65	1045.35	1045.05	1044.80	296+00	1044.80	1045.05	1044.75	1044.45	1044.03
1045.40	1045.81	1045.46	1045.11	1044.86	296+25	1044.86	1045.11	1044.76	1044.41	1043.99
1045.60	1045.97	1045.57	1045.17	1044.92	296+50	1044.95	1045.17	1044.77	1044.37	1043.95
1045.73	1046.07	1045.64	1045.21	1044.96	296+66	1045.00	1045.21	1044.78	1044.35	1043.93
				FULL	SUPER					
PG +0.45	PG +0.86	PG +0.43		PG -0.50		PG -0.41		PG -0.43	PG -0.86	PG -1.36
PG +0.45	PG +0.86	PG +0.43		PG -0.50		PG -0.41		PG -0.43	PG -0.86	PG -1.36
				FULL	SUPER					
1046.59	1046.93	1046.50	1046.07	1045.82	324+25	1045.86	1046.07	1045.64	1045.21	1044.79
1046.51	1046.89	1046.51	1046.13	1045.88	324+50	1045.90	1046.13	1045.75	1045.37	1044.95
1046.43	1046.85	1046.52	1046.19	1045.94	324+75	1045.94	1046.19	1045.86	1045.53	1045.11
1046.41	1046.83	1046.54	1046.25	1046.00	PT 324+98.06	1046.00	1046.25	1045.96	1045.67	1045.25
1046.39	1046.81	1046.53	1046.25	1046.00	325+00	1046.00	1046.25	1045.97	1045.69	1045.27
1046.36	1046.78	1046.55	1046.32	1046.07	325+25	1046.07	1046.32	1046.09	1045.86	1045.44
1046.34	1046.76	1046.57	1046.38	1046.13	325+45	1046.13	1046.38	1046.19	1046.00	1045.58
1046.33	1046.75	1046.58	1046.39	1046.14	325+50	1046.14	1046.39	1046.22	1046.03	1045.61
1046.32	1046.74	1046.67	1046.48	1046.23	325+75	1046.23	1046.48	1046.41	1046.22	1045.80
1046.32	1046.74	1046.74	1046.55	1046.30	325+93	1046.30	1046.55	1046.55	1046.36	1045.94
1046.32	1046.74	1046.77	1046.58	1046.33	326+00	1046.33	1046.58	1046.61	1046.42	1046.00
1046.32	1046.74	1046.87	1046.68	1046.43	326+25	1046.43	1046.68	1046.81	1046.62	1046.20
1046.34	1046.76	1046.95	1046.76	1046.51	326+41	1046.51	1046.76	1046.95	1046.76	1046.34

CURVE No. 4

SE = 0.036 Dc = 01° 00' LT.

OUTSIDE EDGE PAVED SHOULDER	EDGE PAVEMENT	CENTER	P. G. EDGE PAVEMENT	MEDIAN EDGE PAVED SHOULDER	STATION	MEDIAN EDGE PAVED SHOULDER	P. G. EDGE PAVEMENT	CENTER	EDGE PAVEMENT	OUTSIDE EDGE PAVED SHOULDER
1059.69	1060.11	1060.30	1060.11	1059.86	344+50	1059.86	1060.11	1060.30	1060.11	1059.69
1059.46	1059.88	1060.07	1059.98	1059.73	344+75	1059.73	1059.98	1060.17	1060.08	1059.66
1059.25	1059.67	1059.86	1059.86	1059.61	344+98	1059.61	1059.86	1060.05	1060.05	1059.63
1059.23	1059.65	1059.84	1059.85	1059.60	345+00	1059.60	1059.85	1060.04	1060.05	1059.63
1058.98	1059.40	1059.59	1059.70	1059.45	345+25	1059.45	1059.70	1059.89	1060.00	1059.58
1058.77	1059.19	1059.38	1059.57	1059.32	346+46	1059.32	1059.57	1059.76	1059.95	1059.53
1058.73	1059.15	1059.35	1059.55	1059.30	345+50	1059.30	1059.55	1059.75	1059.95	1059.53
1058.46	1058.88	1059.13	1059.38	1059.13	345+75	1059.13	1059.38	1059.63	1059.88	1059.46
1058.36	1058.78	1059.05	1059.32	1059.07	PC 345+83.02	1059.07	1059.32	1059.59	1059.86	1059.44
1058.18	1058.60	1058.90	1059.20	1058.95	346+00	1058.95	1059.20	1059.50	1059.80	1059.38
1057.89	1058.31	1058.66	1059.01	1058.76	346+25	1058.76	1059.01	1059.36	1059.71	1059.30
1057.59	1058.01	1058.41	1058.81	1058.59	346+50	1058.56	1058.81	1059.21	1059.61	1059.24
1057.39	1057.81	1058.24	1058.67	1058.46	346+66	1058.42	1058.67	1059.10	1059.53	1059.19
				FULL	SUPER					
PG -1.36	PG -0.86	PG +.043		PG -.041		PG -0.50		PG +0.43	PG +0.86	PG +0.45
PG -1.36	PG -0.86	PG -0.43		PG -0.41		PG -0.50		PG +0.43	PG +0.86	PG +0.45
				FULL	SUPER					
1033.03	1033.45	1033.88	1034.31	1034.10	380+09	1034.06	1034.31	1034.74	1035.17	1034.83
1033.05	1033.47	1033.87	1034.27	1034.05	380+25	1034.02	1034.27	1034.67	1035.07	1034.70
1033.09	1033.51	1033.86	1034.21	1033.96	380+50	1033.96	1034.21	1034.56	1034.91	1034.50
1033.13	1033.55	1033.85	1034.15	1033.90	380+75	1033.90	1034.15	1034.45	1034.75	1034.33
1033.15	1033.57	1033.85	1034.13	1033.88	PT 380+84.82	1033.88	1034.13	1034.41	1034.69	1034.27
1033.17	1033.59	1033.84	1034.09	1033.84	381+00	1033.84	1034.09	1034.34	1034.59	1034.17
1033.21	1033.63	1033.83	1034.03	1033.78	381+25	1033.78	1034.03	1034.23	1034.43	1034.01
1033.22	1033.64	1033.83	1034.02	1033.77	381+29	1033.77	1034.02	1034.21	1034.40	1033.98
1033.25	1033.67	1033.86	1033.97	1033.72	381+50	1033.72	1033.97	1034.16	1034.27	1033.85
1033.29	1033.71	1033.90	1033.91	1033.66	381+75	1033.66	1033.91	1034.10	1034.11	1033.69
1033.30	1033.72	1033.91	1033.91	1033.66	381+77	1033.66	1033.91	1034.10	1034.10	1033.68
1033.33	1033.75	1033.94	1033.85	1033.60	382+00	1033.60	1033.85	1034.04	1033.95	1033.53
1033.37	1033.79	1033.98	1033.79	1033.54	382+25	1033.54	1033.79	1033.98	1033.79	1033.37

REVISED 10-99 BY GFR
CALCULATED J.L.G. CHECKED W.G.Y.

SUPERELEVATION TABLES - CURVE 3 & 4

FAY-35-2.57

DRAINAGE SUB-SUMMARY

ITEM	SHEET NUMBERS 285	313	344	ITEM	GRAND TOTAL	UNIT	DESCRIPTION
301	9			301	9	C.Y.	BITUMINOUS AGGREGATE BASE, PG64-22
448	3			448	3	C.Y.	ASPHALT CONCRETE, INTERMEDIATE COURSE, TYPE 2, PG64-22
448	2			448	2	C.Y.	ASPHALT CONCRETE, SURFACE COURSE, TYPE 1, PG64-22
601	200			601	148	C.Y.	ROCK CHANNEL PROTECTION, TYPE B, W/FILTER
601	73	84		601	157	C.Y.	ROCK CHANNEL PROTECTION, TYPE C, W/FILTER
602	47.6	9.7		602	57.3	C.Y.	CONCRETE MASONRY
603			2839	603	2839	L.F.	6" CONDUIT, TYPE B
603			4027	603	4027	L.F.	6" CONDUIT, TYPE F, 707.41
603	70			603	70	L.F.	15" CONDUIT, TYPE A, 706.01, 707.01(.109), 707.01 ALUMINUM COATED(.079) OR 707.21
603	64			603	64	L.F.	15" CONDUIT, TYPE A, 706.01, OR 18" CONDUIT, TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.079) OR 707.21
603		2262		603	2262	L.F.	15" CONDUIT, TYPE B
603		407		603	407	L.F.	15" CONDUIT, TYPE F
603	89			603	89	L.F.	18" CONDUIT, TYPE A, 706.01 OR 706.02 OR 21" CONDUIT, TYPE A, 707.01 OR 707.21
603	80			603	80	L.F.	18" CONDUIT, TYPE A, 706.01 OR 706.02 OR 24" CONDUIT, TYPE A, 707.01
603		90		603	90	L.F.	18" CONDUIT, TYPE B
603	85			603	85	L.F.	21" CONDUIT, TYPE A, 706.02 OR 706.08 OR 27" CONDUIT, TYPE A, 707.05
603		875		603	875	L.F.	21" CONDUIT, TYPE C
603	122			603	122	L.F.	24" CONDUIT, TYPE A, 706.02, 707.01(.079), 707.01 ALUMINUM COATED(.064) 707.05, OR 707.21(.060)
603	76			603	76	L.F.	24" CONDUIT, TYPE A, 706.02, OR 706.08 OR 27" CONDUIT, TYPE A, 707.05
603	199			603	199	L.F.	24" CONDUIT, TYPE A, 706.02, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079) OR 707.21(.105)
603	150			603	150	L.F.	24" CONDUIT, TYPE A, 706.02, 1500 D-LOAD, OR 707.01(.079), 707.01 ALUMINUM COATED(.064), 707.05 OR 707.21(.060)
603		186		603	186	L.F.	24" CONDUIT, TYPE B
603	63			603	63	L.F.	30" CONDUIT, TYPE A, 706.02
603	196			603	196	L.F.	30" CONDUIT, TYPE A, 706.02, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.075)
603	223			603	223	L.F.	30" CONDUIT, TYPE A, 706.02, 1250 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603	157			603	157	L.F.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, OR 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.075)
603	180			603	180	L.F.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, OR 36" CONDUIT, TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603	114			603	114	L.F.	30" CONDUIT, TYPE A, 706.02, 1750 D-LOAD, OR 42" CONDUIT, TYPE A, 707.01 ALUMINUM COATED(.109), 707.05, OR 707.21(.105)
603	210			603	210	L.F.	30" CONDUIT, TYPE A, 706.02, OR 48" CONDUIT, TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079) OR, 707.21(.105)
603		183		603	183	L.F.	30" CONDUIT, TYPE B
603		1500		603	1500	L.F.	30" CONDUIT, TYPE C
603	100			603	100	L.F.	36" CONDUIT, TYPE A, 706.02, OR 706.08, OR 48" CONDUIT, TYPE A, 707.05
603	206			603	206	L.F.	36" CONDUIT, TYPE A, 706.02, OR 54" CONDUIT, TYPE A, 707.02(.109), 707.02 FIELD PAVED (.064), 707.02 ALUMINUM COATED(.064), 707.07(.079), OR 707.22
603	223			603	223	L.F.	36" CONDUIT, TYPE A, 706.02, 1500 D-LOAD, OR 48" CONDUIT, TYPE A, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603	224			603	224	L.F.	36" CONDUIT, TYPE A, 706.02, 2000 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)
603	268			603	268	L.F.	36" CONDUIT, TYPE A, 706.02, 2500 D-LOAD, 707.01(.109), 707.01 ALUMINUM COATED(.079), 707.05(.079), OR 707.21(.105)
603		720		603	720	L.F.	36" CONDUIT, TYPE C
603	263			603	263	L.F.	42" CONDUIT, TYPE A, 706.02, 2500 D-LOAD, OR 66" CONDUIT, TYPE A, 707.02 FIELD PAVED (.064), 707.02(.109), 707.02 ALUMINUM COATED(.064), 707.04, 707.07(.079), OR 707.22
603	200			603	200	L.F.	48" CONDUIT, TYPE A, 706.02, OR 66" CONDUIT, TYPE A, 707.02 FIELD PAVED (.064), 707.02(.109), 707.02 ALUMINUM COATED(.079), 707.04, 707.07(.079), OR 707.22
603	196			603	196	L.F.	54" CONDUIT, TYPE A, 706.02
603	211			603	211	L.F.	60" CONDUIT, TYPE A, 706.02
603	224			603	224	L.F.	22"x 34" CONDUIT, TYPE A, 706.04 HE-II
603	219			603	219	L.F.	48"x 76" CONDUIT, TYPE A, 706.04 HE-I
604		4		604	4	EACH	CATCH BASIN NO. 3A
604		23		604	23	EACH	CATCH BASIN NO. 4
604		1		604	1	EACH	CATCH BASIN NO. 4A
604		3		604	3	EACH	CATCH BASIN NO. 8
604		9		604	9	EACH	CATCH BASIN NO. 8A
604		2		604	2	EACH	CATCH BASIN NO. 2-2B
604			68	604	68	EACH	PRECAST REINFORCED CONCRETE OUTLET
605			4516	605	4516	L.F.	4" SHALLOW PIPE UNDERDRAIN
605			101,045	605	101,045	L.F.	4" SHALLOW PIPE UNDERDRAIN, WITH FABRIC WRAP, AS PER PLAN (SEE SHEET 6)
605			98,568	605	98,568	L.F.	6" SHALLOW PIPE UNDERDRAIN
605			9950	605	9950	L.F.	6" DEEP PIPE UNDERDRAIN
605			2782	605	2782	L.F.	6" UNCLASSIFIED PIPE UNDERDRAIN

CALCULATED | CHECKED

DRAINAGE SUB-SUMMARY

FAY-35-2.57

279
522

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&75\40lnewsht\new279.dgn 20-MAR-2001 3:40PM dwise

WEST LANCASTER ROAD C.R.-16

JEFFERSONVILLE-WEST LANCASTER ROAD T.R.-94

CULVERT S-1
STA. 2+06 T.R.94
2.5 AC.

RATTLESNAKE CREEK

15" TILE

5" TILE

MATCH LINE STA. 190+00

H-1 H-7 H-20

C-1 C-2 C-4 C-5 C-6 C-7 C-8 C-9 C-10 C-11 C-12 C-13 C-14 C-15 C-16 C-17 C-18 C-19 C-20

P-7 P-8 P-9 P-10 P-11 P-12 P-13 P-14 P-15 P-16 P-17 P-18

135 140 145 150 155 160 165 170 175 180 185

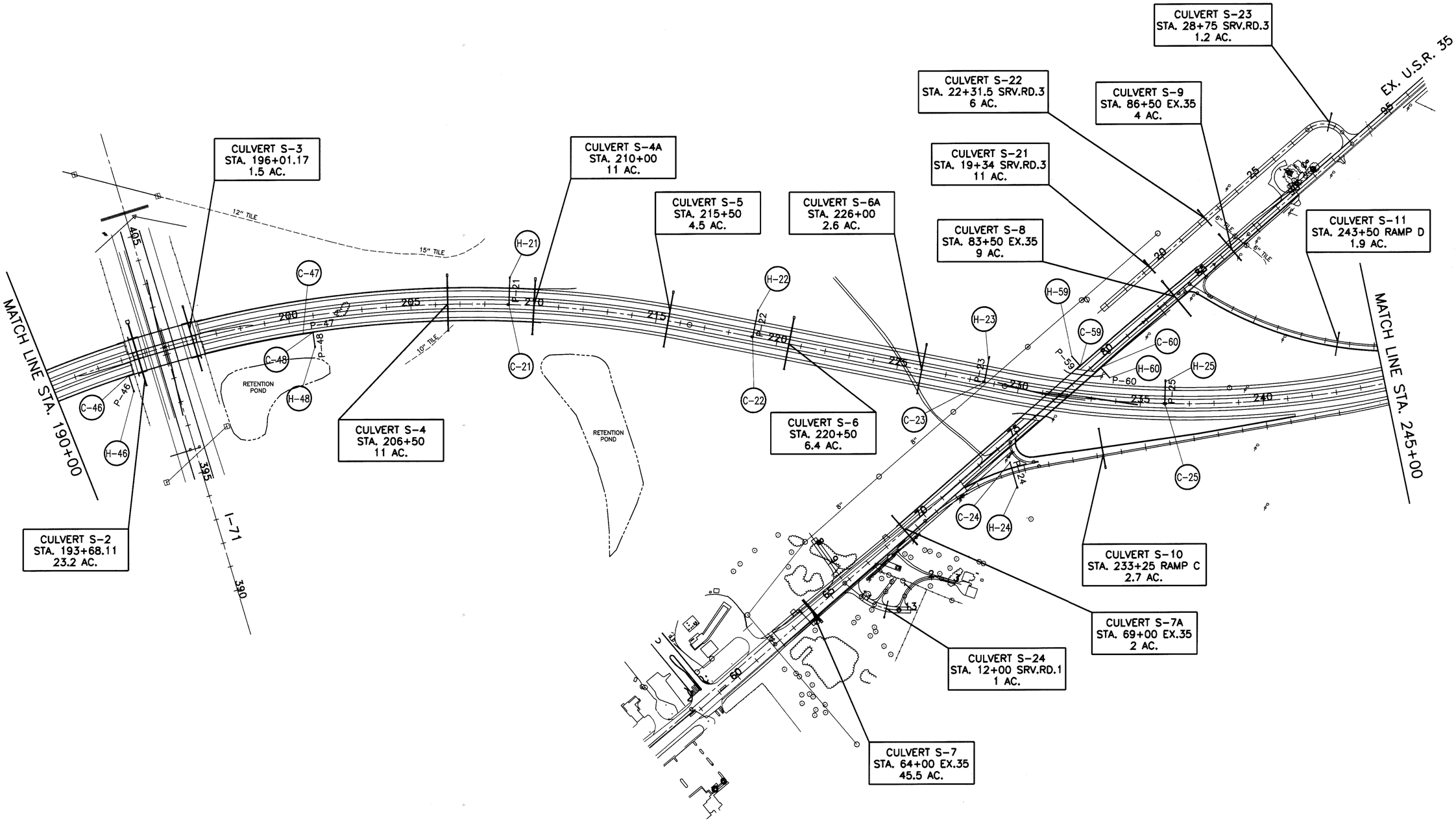
0 100 200 400 HORIZONTAL SCALE IN FEET

CALCULATED | CHECKED

DRAINAGE INDEX

FAY-35-2.57

280
522



0 100 200 400 HORIZONTAL SCALE IN FEET

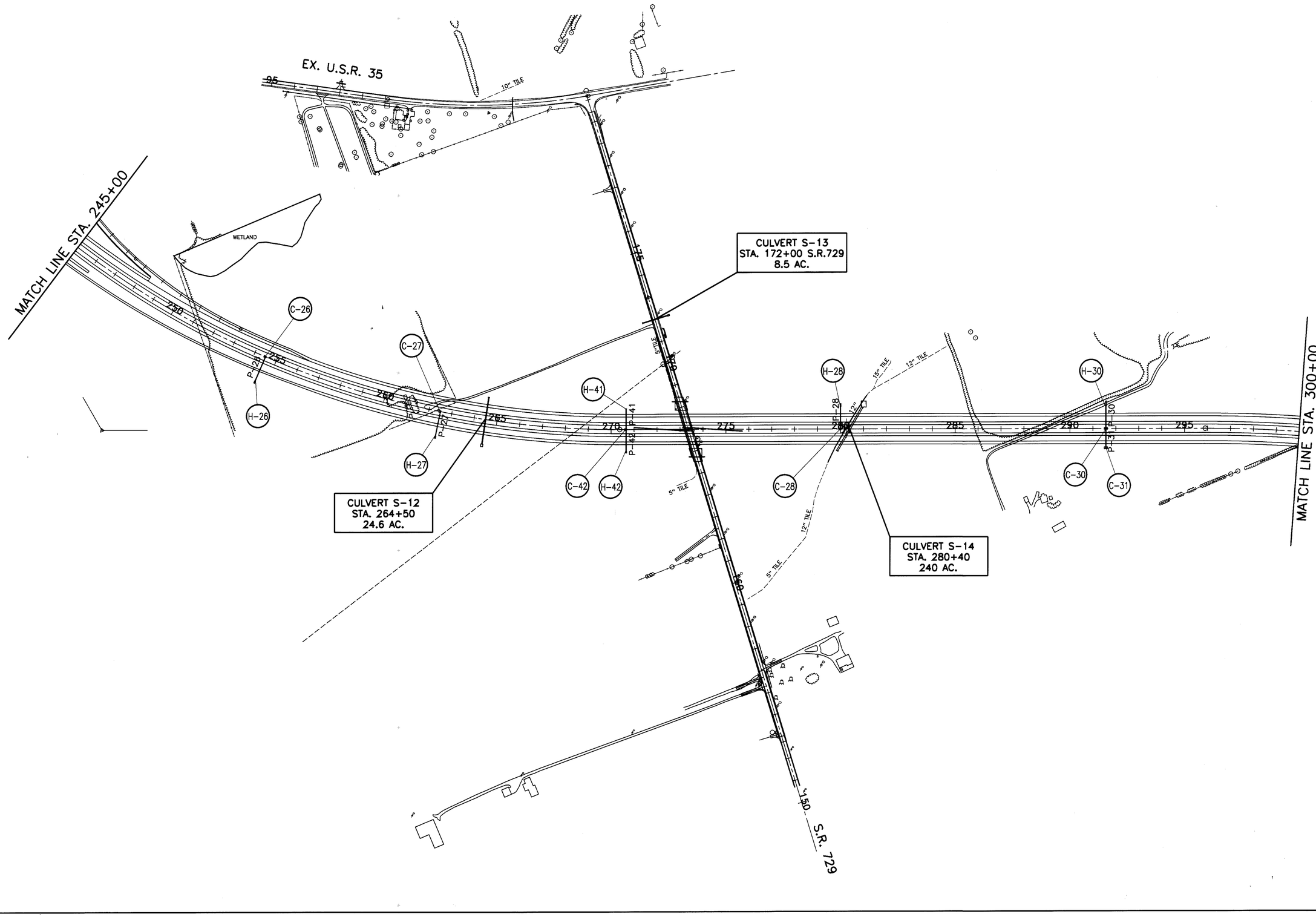
CALCULATED

CHECKED

DRAINAGE INDEX

FAY-35-2.57

281
522



0 100 200 400 HORIZONTAL SCALE IN FEET

CALCULATED

CHECKED

DRAINAGE INDEX

FAY-35-2.57

282
522

0 100 200 400 HORIZONTAL SCALE IN FEET

CALCULATED

CHECKED

DRAINAGE INDEX

FAY-35-2.57

283
522

HORIZONTAL SCALE IN FEET
0 100 200 400

REVISED	CALCULATED	CHECKED
1/00 BY FYC		

DRAINAGE INDEX

FAY-35-2.57

CULVERT SUB-SUMMARY

REFERENCE NUMBER	CULVERT DETAIL SHEET NO.	STATION	301	448	448	601	601	602	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603	603
			BITUMINOUS AGGREGATE BASE, PG64-22	ASPHALT CONCRETE,INTERMEDIATE COURSE, TYPE 2, PG64-22	ASPHALT CONCRETE,SURFACE COURSE, TYPE 1, PG64-22	ROCK CHANNEL PROTECTION, TYPE B W/FILTER	ROCK CHANNEL PROTECTION, TYPE C W/FILTER	CONCRETE MASONRY	15"COND.,TYPE A,706.01,707.01(.109), 707.01ALUMINUM COATED(.079),OR 707.21	15"COND.,TYPE A,706.01 OR 18" COND., TYPE A,707.01(.109),707.01 ALUMINUM COATED(.079),OR 707.21	18"COND.,TYPE A,706.01,OR 706.02 OR 21"COND.,TYPE A,707.01,OR 707.21	18"COND.,TYPE A,706.01,OR 706.02 OR 24"COND.,TYPE A,707.01	21"COND.,TYPE A,706.02,OR 706.08 OR 27"COND.,TYPE A,707.05	24"COND.,TYPE A,706.02,707.01(.079), 707.01 ALUMINUM COATED(.064),707.05, OR 707.21(.060)	24"COND.,TYPE A,706.02,OR 706.08 OR 27"COND.,TYPE A,707.05	24"COND.,TYPE A,706.02,707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079), OR 707.21(.105)	24"COND.,TYPE A,706.02 1500 D-LOAD, OR 707.01(.079),707.01 ALUMINUM COATED(.064),707.05 OR 707.21(.060)	30"COND.,TYPE A,706.02	30"COND.,TYPE A,706.02,707.01(.109), 707.01 ALUMINUM COATED(.064), 707.05(.079),OR 707.21(.105)	30"COND.,TYPE A,706.02 1250 D-LOAD, 707.01(.109),707.01 ALUMINUM COATED (.064),707.05(.079) OR 707.21(.105)	30"COND.,TYPE A,706.02 1500 D-LOAD, 707.01(.109),707.01 ALUMINUM COATED (.064),707.05(.079), OR 707.21(.075)	30"COND.,TYPE A,706.02 1750 D-LOAD, OR 707.01(.109),707.01 ALUMINUM COATED(.064),707.05(.079), OR 707.21(.075)	30"COND.,TYPE A,706.02 1750 D-LOAD,OR 36"COND.,TYPE A,707.01(.109),707.01 ALUMINUM COATED(.064),707.05(.079), OR 707.21(.105)	30"COND.,TYPE A,706.02 1750 D-LOAD,OR 42"COND.,TYPE A,707.01 ALUMINUM COATED(.109),707.05 OR 707.21(.105)	30"COND.,TYPE A,706.02 OR 48" COND., TYPE A,707.01(.109),707.01 ALUMINUM COATED(.064),707.05(.079), OR 707.21(.105)	36"COND.,TYPE A,706.02 OR 706.08 OR 48"COND.,TYPE A,707.05	36"COND.,TYPE A,706.02 OR 54"COND., TYPE A,707.02(.109),707.02 FIELD PAVED (.064),707.02 ALUMINUM COATED(.064),707.07(.079) OR 707.22	36"COND.,TYPE A,706.02 1500 D-LOAD,OR 48"COND.,TYPE A,707.01(.109),707.01 ALUMINUM COATED(.064),707.05(.079), OR 707.21(.105)	36"COND.,TYPE A,706.02 2000 D-LOAD, 707.01(.109),707.01 ALUMINUM COATED (.064),707.05(.079) OR 707.21(.105)	36"COND.,TYPE A,706.02 2500 D-LOAD, 707.01(.109),707.01 ALUMINUM COATED (.079),707.05(.079), OR 707.21(.105)	42"COND.,TYPE A,706.02 2500 D-LOAD, OR 66"COND.,TYPE A,707.02 FIELD PAVED (.064),707.02(.109),707.02 ALUM. COATED(.079),707.04,707.07(.079) OR 707.22	48"COND.,TYPE A,706.02, OR 66"COND., TYPE A,707.02 FIELD PAVED (.064),707.02(.109),707.02 ALUM. COATED (.079),707.04,707.07(.079) OR 707.22	54"COND.,TYPE A,706.02	60"COND.,TYPE A,706.02	22"x34"COND.,TYPE A,706.04 HE-II	48"x76"COND.,TYPE A,706.04 HE-I
			CU.YD.	CU.YD.	CU.YD.	CU.YD.	CU.YD.	CU.YD.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.
S-1	286	STA. 2+06 T.R.94					2	0.8				80																					
S-2	287	STA. 193+68.11				15		4.2																					263			
S-3	288	STA. 196+01.17				3		1.6																				268				
S-4	289	STA. 206+50					8	1.6																			224					
S-4A	290	STA. 210+00					3	2.2																		223						
S-5	291	STA. 215+50					3	1.2											223													
S-6	292	STA. 220+50					4	2.2														210										
S-6A	293	STA. 226+00					2	0.9							199																	
S-7	294	STA. 64+00 EX.35					5	1.6																							224	
S-7A	295	STA. 69+00 EX.35					2	1.0								150																
S-8	296	STA. 83+50 EX.35					3	1.6													180											
S-9	297	STA. 86+50 EX.35					2	1.2						122																		
S-10	298	STA. 233+25 RAMP C					5	1.2												157												
S-11	299	STA. 243+50 RAMP D					2	0.8			89																					
S-12	300	STA. 264+50					6	2.7																	206							
S-13	301	STA. 172+00 S.R.729					3	1.2														114										
S-14	302	STA. 280+40				51		2.7																								219
S-15	303	STA. 310+00				78		3.9																							211	
S-16	304	STA. 316+50					5	1.2											196													
S-17	305	STA. 83+19.55 C.R.85	4	1	1		2	1.1					85																			
S-18	306	STA. 86+86.38 C.R.85	5	2	1		4	2.2																100								
S-19	307	STA. 368+00				23		2.7																						196		
S-20	308	STA. 389+75				30		4.2																					200			
S-21	309	STA. 19+34 SRV.RD.3					3	1.2									63															
S-22	310	STA. 22+31.5 SRV.RD.3					4	1.1							76																	
S-23	311	STA. 28+75 SRV.RD.3					3	0.6	70																							
S-24	312	STA. 12+00 SRV.RD.1					2	0.7		64																						
TOTALS TO SHEET 279			9	3	2	200	73	47.6	70	64	89	80	85	122	76	199	150	63	196	223	157	180	114	210	100	206	223	224	268	263	200	196	211	224	219

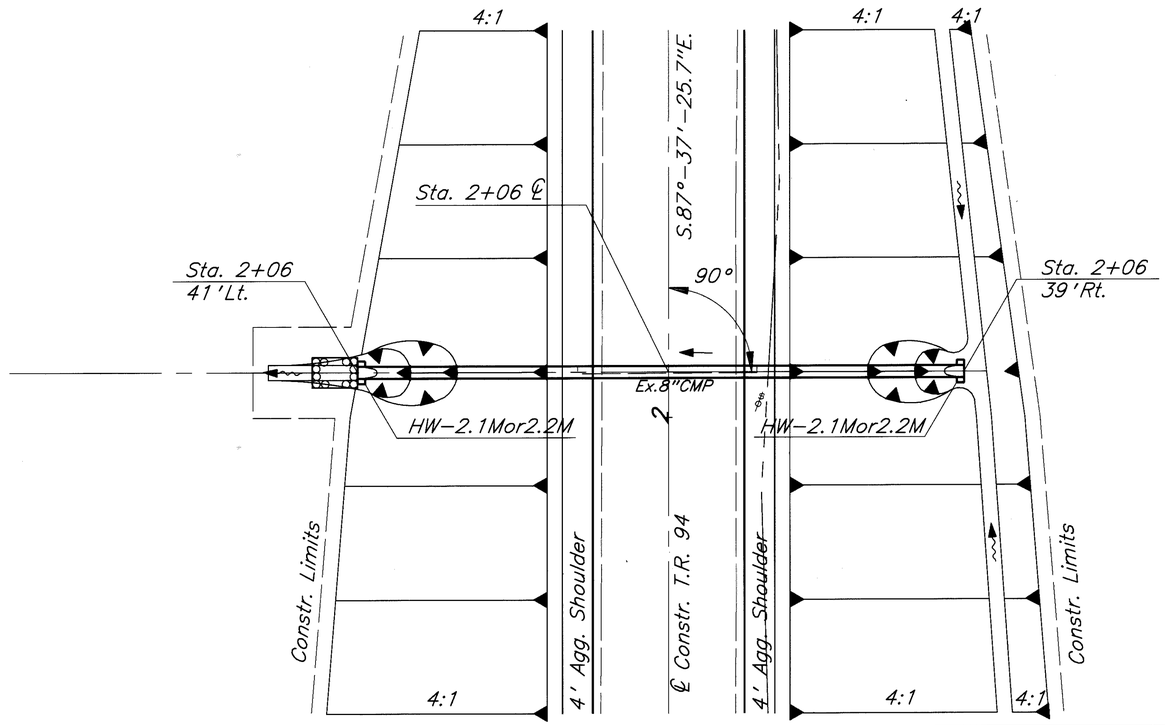
CALCULATED

CHECKED

CULVERT SUB-SUMMARY

FAY-35-2.57

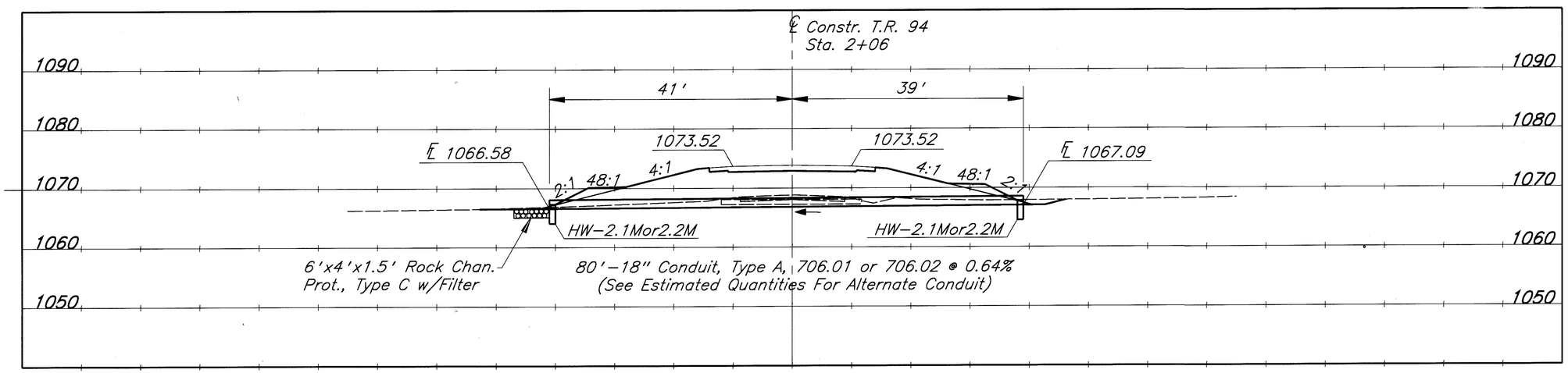
HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=2.5 Ac.
Q 10	= 7 cfs
Q 100	=10 cfs
HW 10	= 1068.7
HW 100	= 1069.1
V 10	=5.7 fps
V 100	=5.7 fps



NOTE: DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE PAVED SHOULDER DIMENSIONS ON THE CULVERT SHEETS WERE REVISED, BUT THE SHOULDERS WERE NOT REDRAWN TO SCALE.

ESTIMATED QUANTITIES *		
601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 0.8 Cu.Yd.
603	18" Conduit, Type A, 706.01 or 706.02 or 24" Conduit, Type A, 707.01	= 80 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CULVERT DETAIL S-1
T.R. 94 – STA. 2 + 06

HORIZONTAL SCALE IN FEET
0 5 10 20

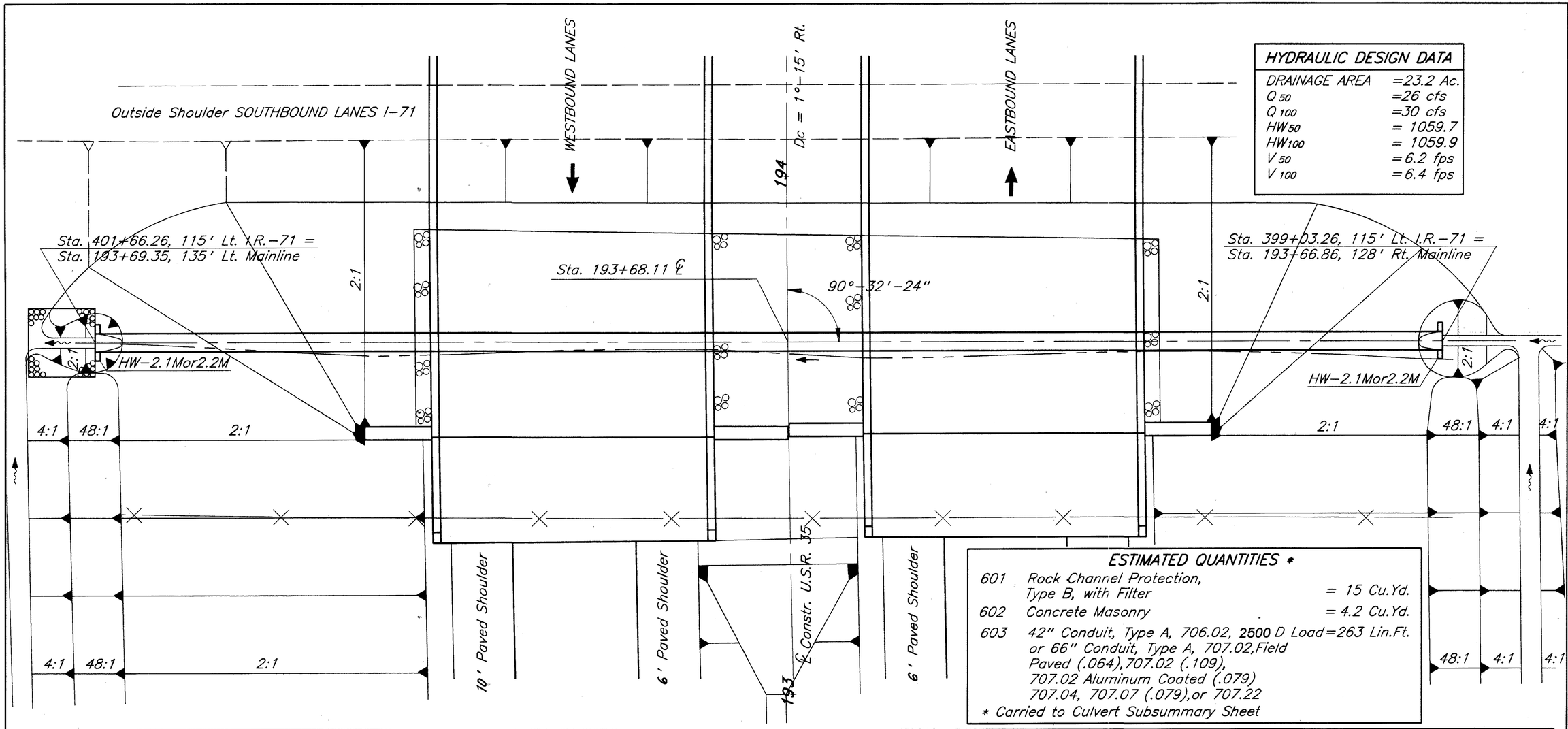
CALCULATED | CHECKED

REVISED 10-99 BY FYC

FAY-35-2.57

286
522

S-1



HYDRAULIC DESIGN DATA

DRAINAGE AREA	=23.2 Ac.
Q_{50}	=26 cfs
Q_{100}	=30 cfs
HW_{50}	= 1059.7
HW_{100}	= 1059.9
V_{50}	=6.2 fps
V_{100}	=6.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type B, with Filter	= 15 Cu.Yd.
602	Concrete Masonry	= 4.2 Cu.Yd.
603	42" Conduit, Type A, 706.02, 2500 D Load or 66" Conduit, Type A, 707.02, Field Paved (.064), 707.02 (.109), 707.02 Aluminum Coated (.079) 707.04, 707.07 (.079), or 707.22	=263 Lin.Ft.

* Carried to Culvert Subsummary Sheet

NOTE : DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE PAVED SHOULDER DIMENSIONS ON THE CULVERT SHEETS HAVE BEEN REVISED, BUT THE PAVED SHOULDERS WERE NOT REDRAWN TO SCALE.

S-2

HORIZONTAL SCALE IN FEET: 0, 5, 10, 20

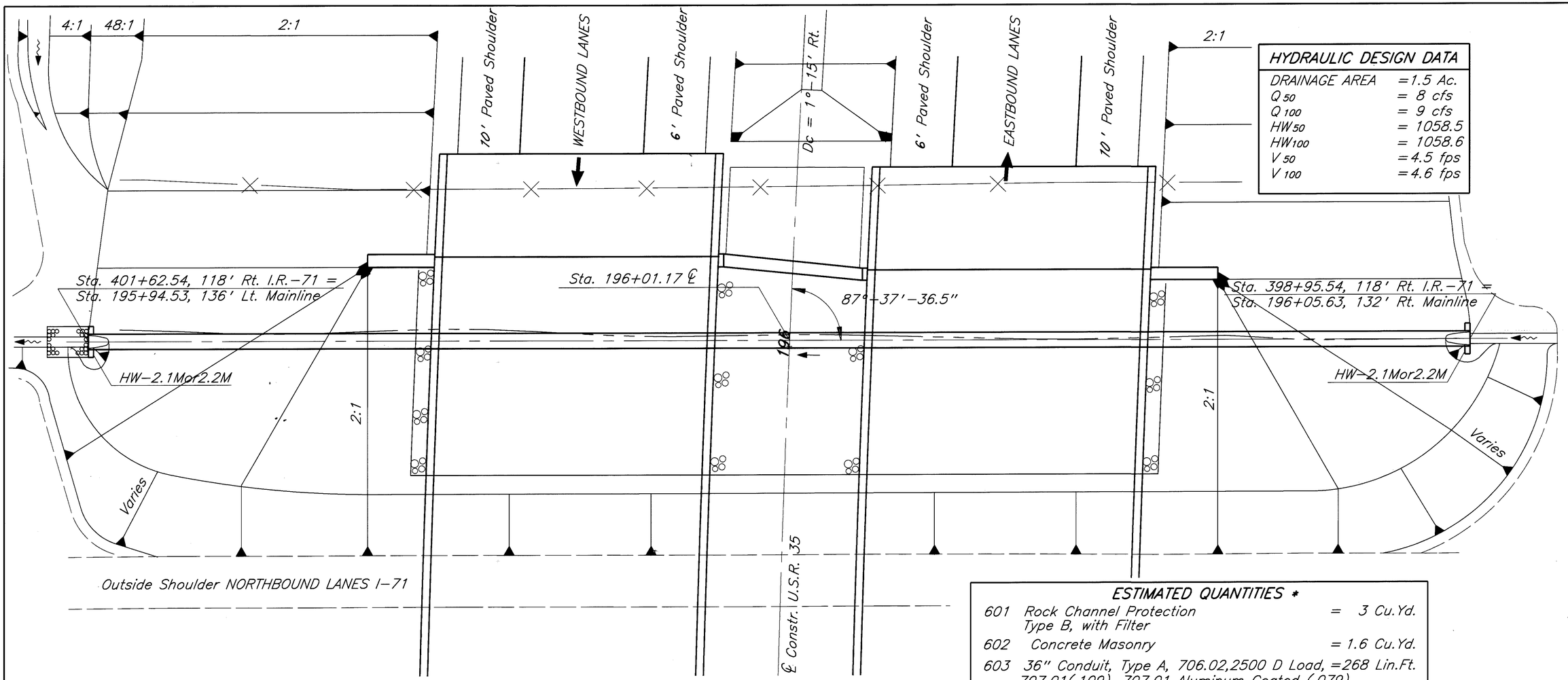
CALCULATED | CHECKED

REVISED 10-99 BY FYC

CULVERT DETAIL S-2
U.S.R. 35 - STA. 193 + 68.11

FAY-35-2.57

287
522



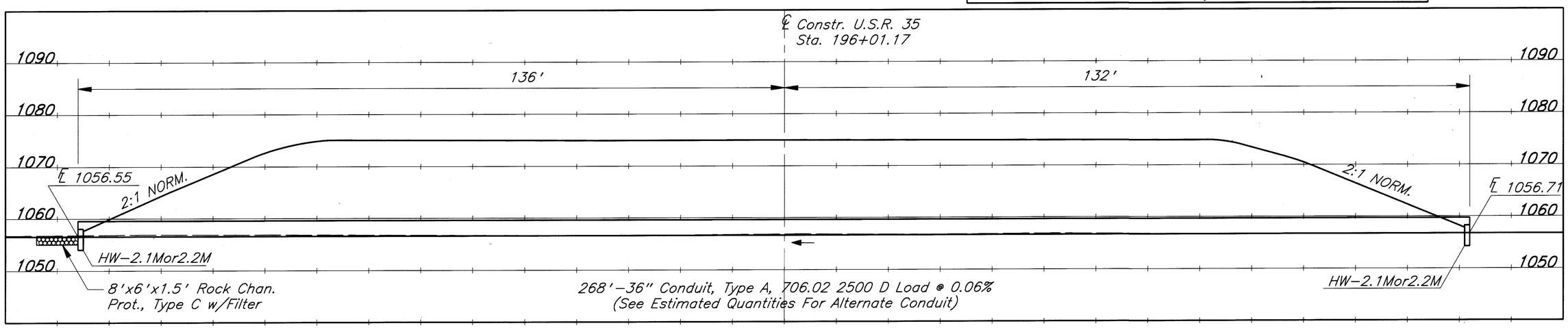
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=1.5 Ac.
Q_{50}	= 8 cfs
Q_{100}	= 9 cfs
HW_{50}	= 1058.5
HW_{100}	= 1058.6
V_{50}	=4.5 fps
V_{100}	=4.6 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection Type B, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 1.6 Cu.Yd.
603	36" Conduit, Type A, 706.02,2500 D Load, 707.01(.109), 707.01 Aluminum Coated (.079), 707.05(.079), or 707.21(.105)	=268 Lin.Ft.

* Carried to Culvert Subsummary Sheet



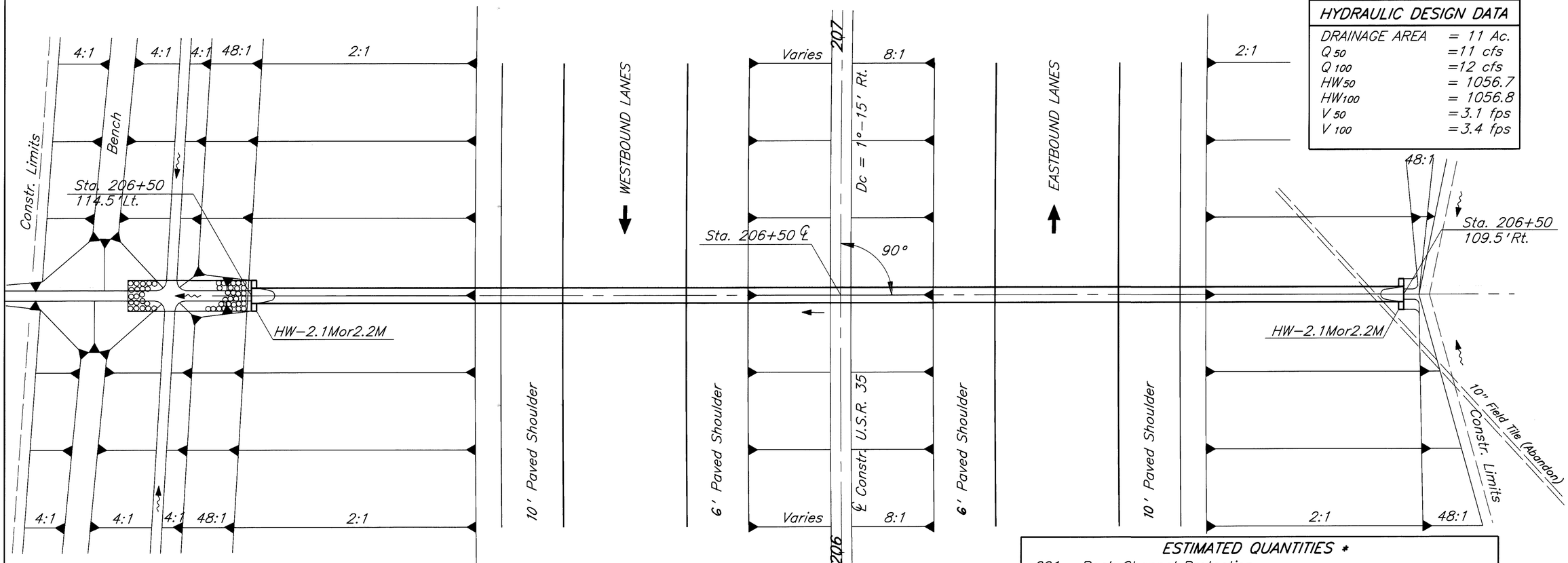
S-3

REVISED 10-99 BY FYC
CALCULATED
CHECKED
0 5 10 20 HORIZONTAL SCALE IN FEET

CULVERT DETAIL S-3
U.S.R. 35 – STA. 196 + 01.17

FAY-35-2.57

288
522



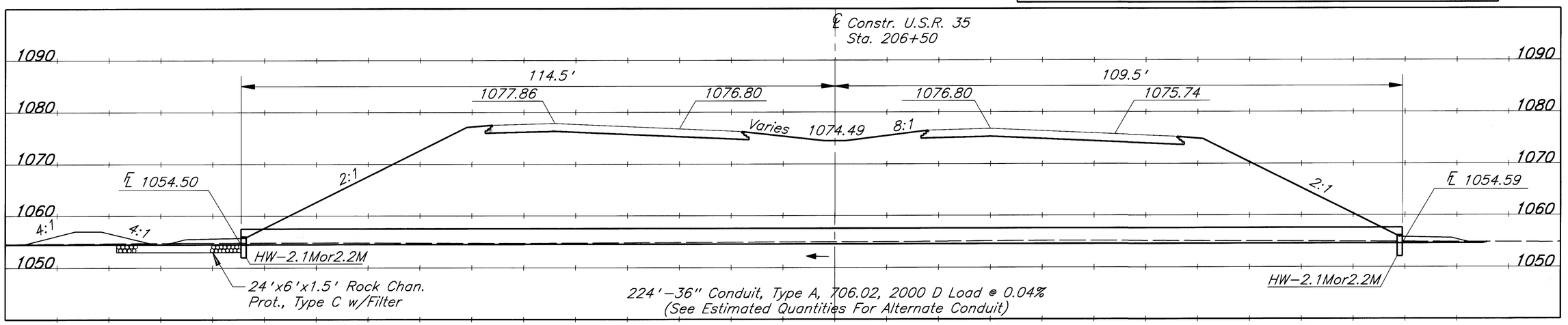
HYDRAULIC DESIGN DATA

DRAINAGE AREA	= 11 Ac.
Q_{50}	=11 cfs
Q_{100}	=12 cfs
HW_{50}	= 1056.7
HW_{100}	= 1056.8
V_{50}	=3.1 fps
V_{100}	=3.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 8 Cu.Yd.
602	Concrete Masonry	= 1.6 Cu.Yd.
603	36" Conduit, Type A, 706.02, 2000 D Load, 707.01(.109), 707.01 Aluminum Coated (.064), 707.05(.079), or 707.21(.105)	=224 Lin.Ft.

* Carried to Culvert Subsummary Sheet



S–4

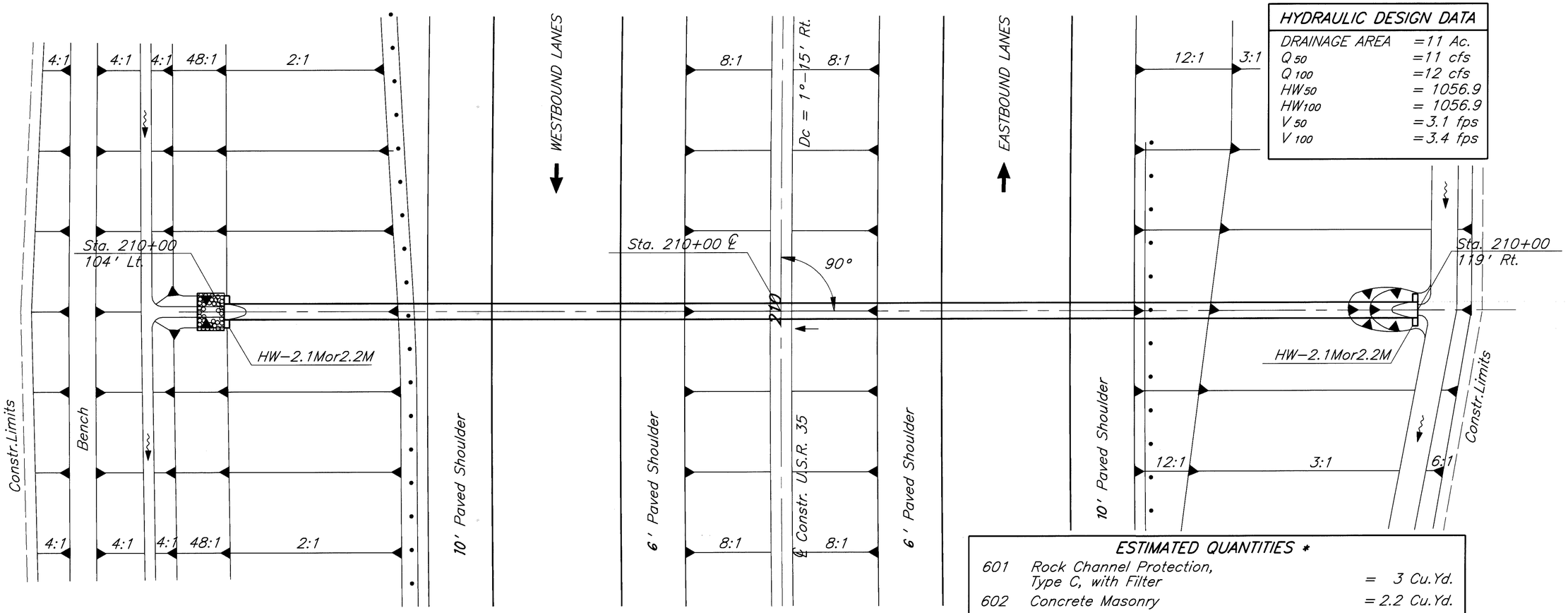
CULVERT DETAIL S–4
U.S.R. 35 – STA. 206 + 50

HORIZONTAL SCALE IN FEET 0 5 10 20

CALCULATED | CHECKED
REVISED 10-99 BY FYC

FAY–35–2.57

289
522



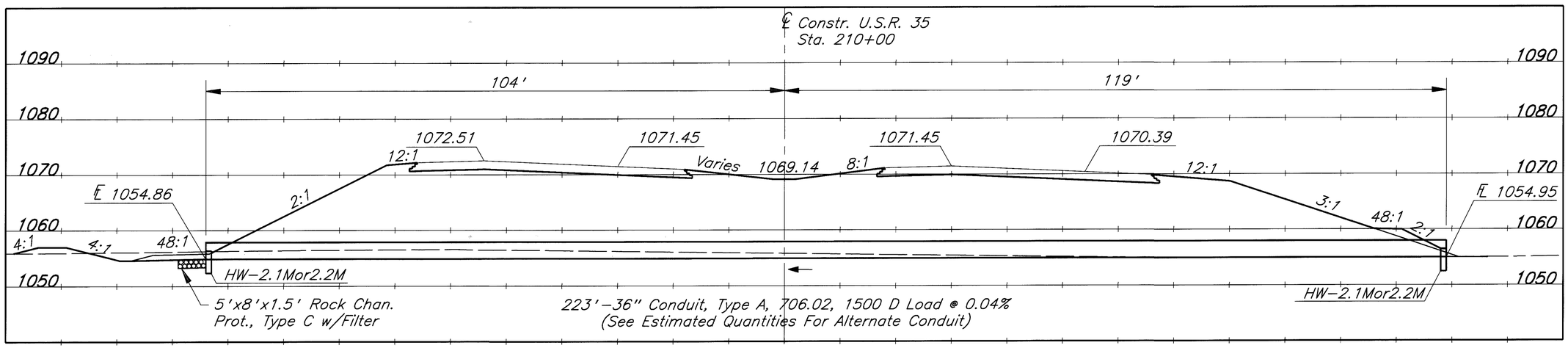
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=11 Ac.
Q_{50}	=11 cfs
Q_{100}	=12 cfs
HW_{50}	= 1056.9
HW_{100}	= 1056.9
V_{50}	=3.1 fps
V_{100}	=3.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 2.2 Cu.Yd.
603	36" Conduit, Type A, 706.02, 1500 D Load, or 48" Conduit, Type A, 707.01(.109), 707.01 Aluminum Coated(.064), 707.05 (.079),or 707.21(.105)	=223 Lin.Ft.

* Carried to Culvert Subsummary Sheet



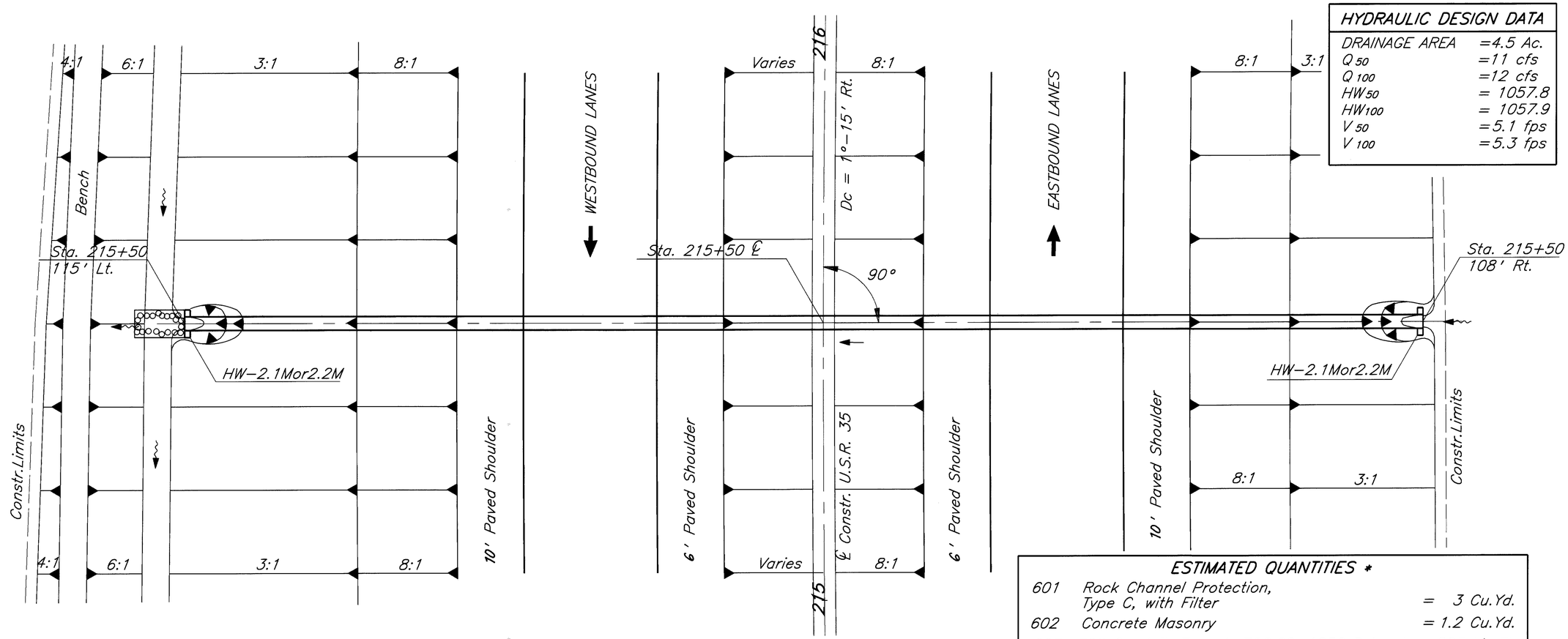
S–4A

REVISED 10-99 BY FYC
CALCULATED
CHECKED
HORIZONTAL SCALE IN FEET 0 5 10 20

CULVERT DETAIL S–4A
U.S.R. 35 – STA. 210 + 00

FAY–35–2.57

290
522

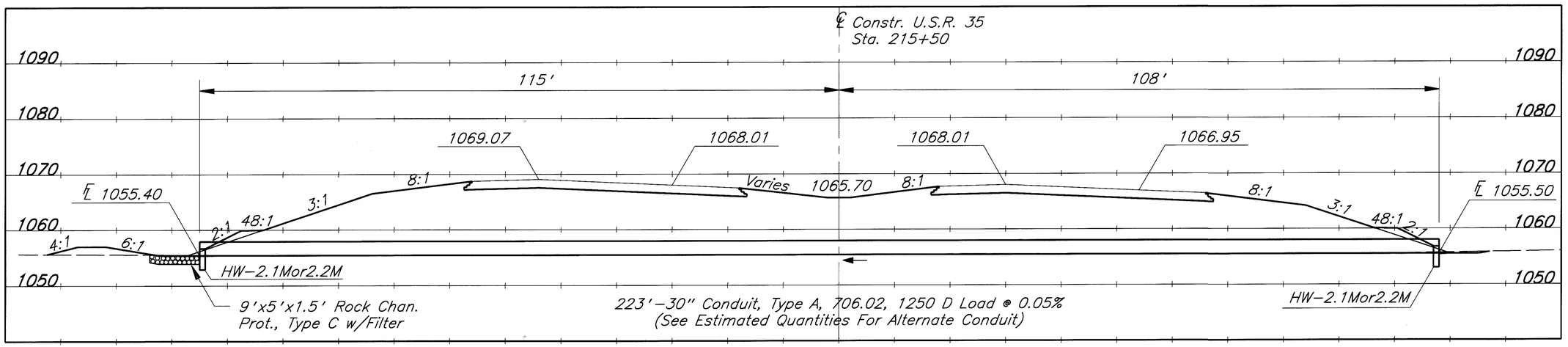


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=4.5 Ac.
Q_{50}	=11 cfs
Q_{100}	=12 cfs
HW_{50}	= 1057.8
HW_{100}	= 1057.9
V_{50}	=5.1 fps
V_{100}	=5.3 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	30" Conduit, Type A, 706.02, 1250 D Load, 707.01(.109), 707.01 Aluminum Coated(.064), 707.05(.079), or 707.21(.105)	=223 Lin.Ft.

* Carried to Culvert Subsummary Sheet



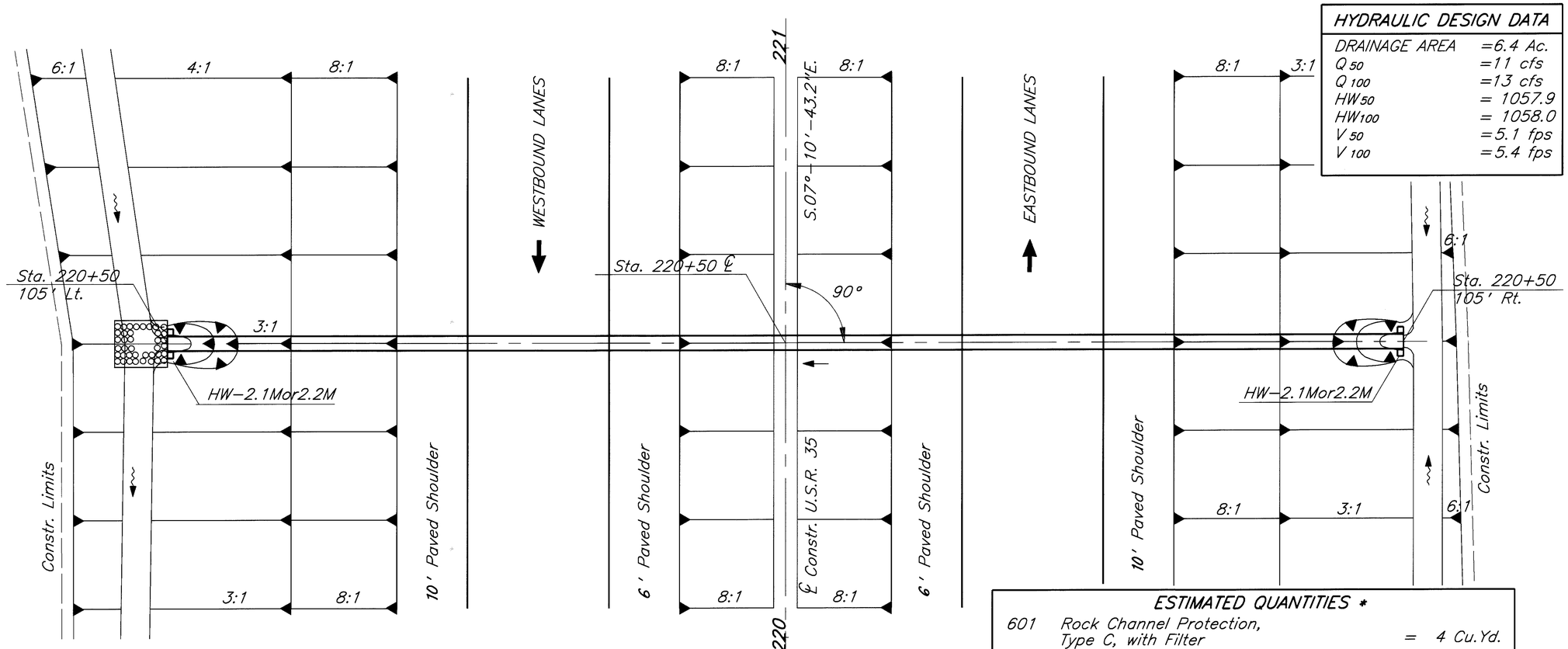
CULVERT DETAIL S-5
U.S.R. 35 - STA. 215 + 50

REVISED 10-99 BY FYC
CALCULATED
CHECKED
HORIZONTAL SCALE IN FEET 0 5 10 20

FAY-35-2.57

S-5

291
522



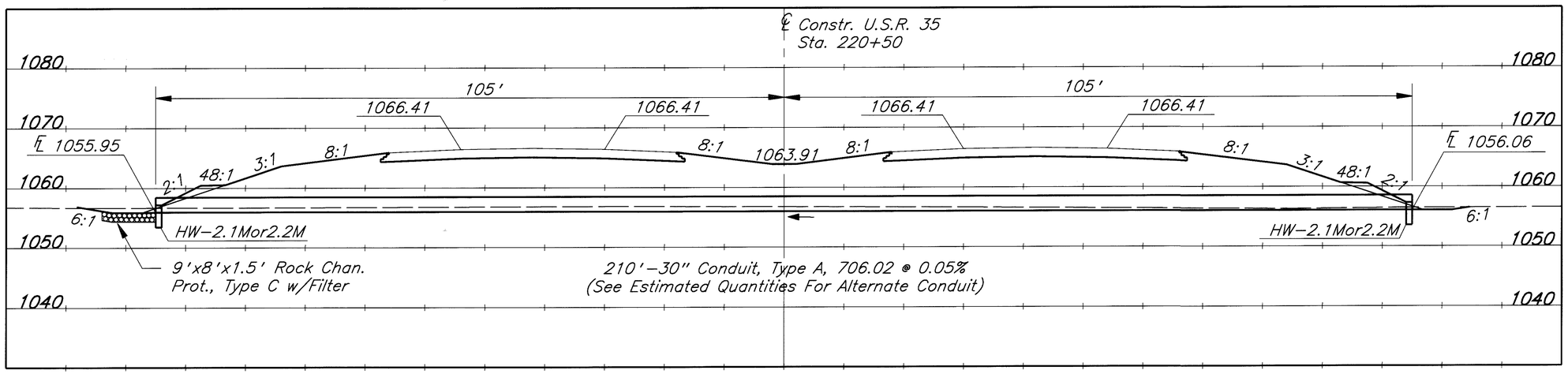
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=6.4 Ac.
Q_{50}	=11 cfs
Q_{100}	=13 cfs
HW_{50}	= 1057.9
HW_{100}	= 1058.0
V_{50}	=5.1 fps
V_{100}	=5.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 4 Cu.Yd.
602	Concrete Masonry	= 2.2 Cu.Yd.
603	30" Conduit, Type A, 706.02 or 48" Conduit, Type A, 707.01(.109), 707.01 Aluminum Coated(.064), 707.05(.079), or 707.21(.105)	=210 Lin.Ft.

* Carried to Culvert Subsummary Sheet



REVISED 10-99 BY FYC | CALCULATED | CHECKED

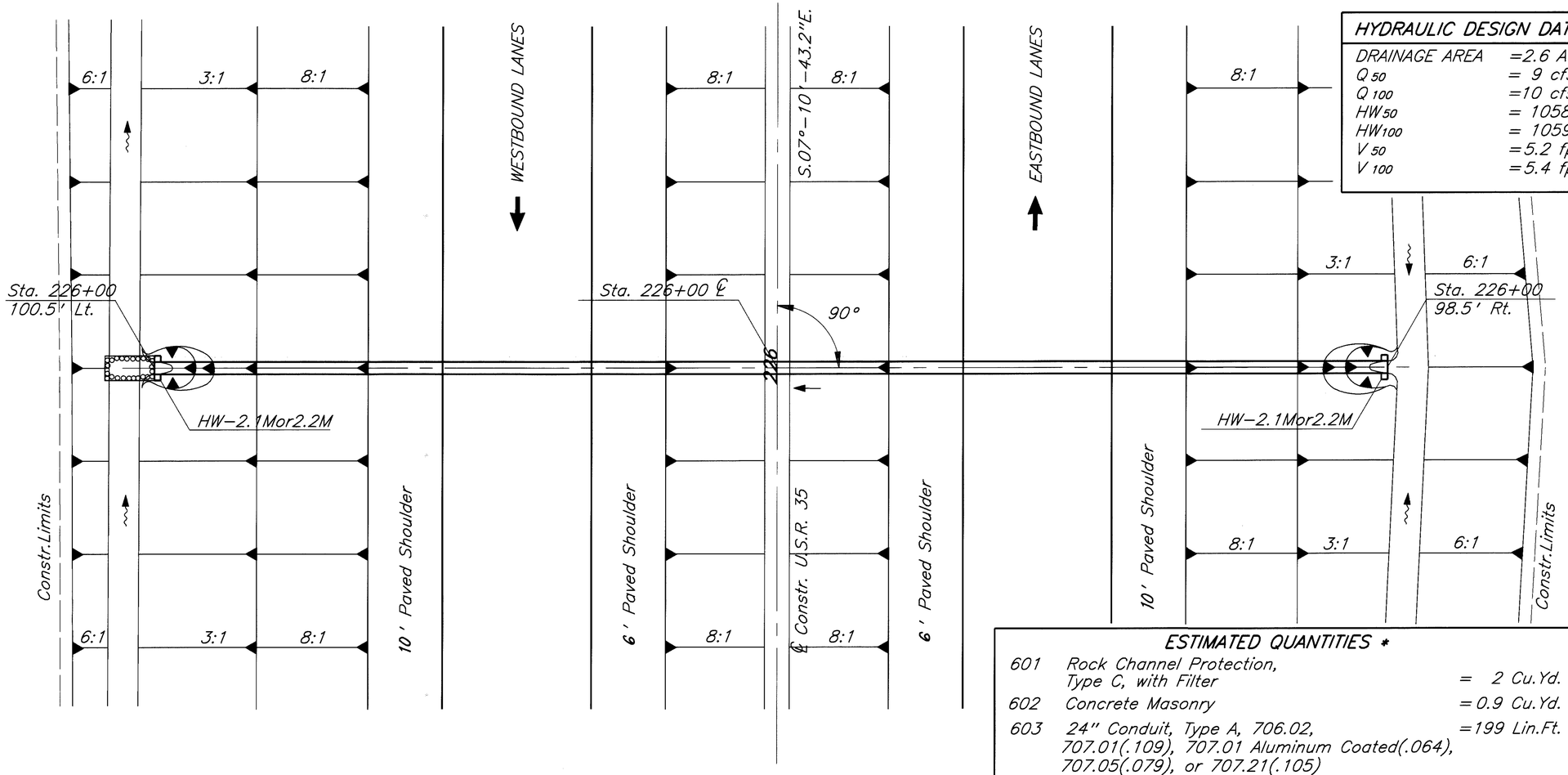
HORIZONTAL SCALE IN FEET 0 5 10 20

CULVERT DETAIL S-6
U.S.R. 35 – STA. 220 + 50

FAY-35-2.57

S-6

292
522



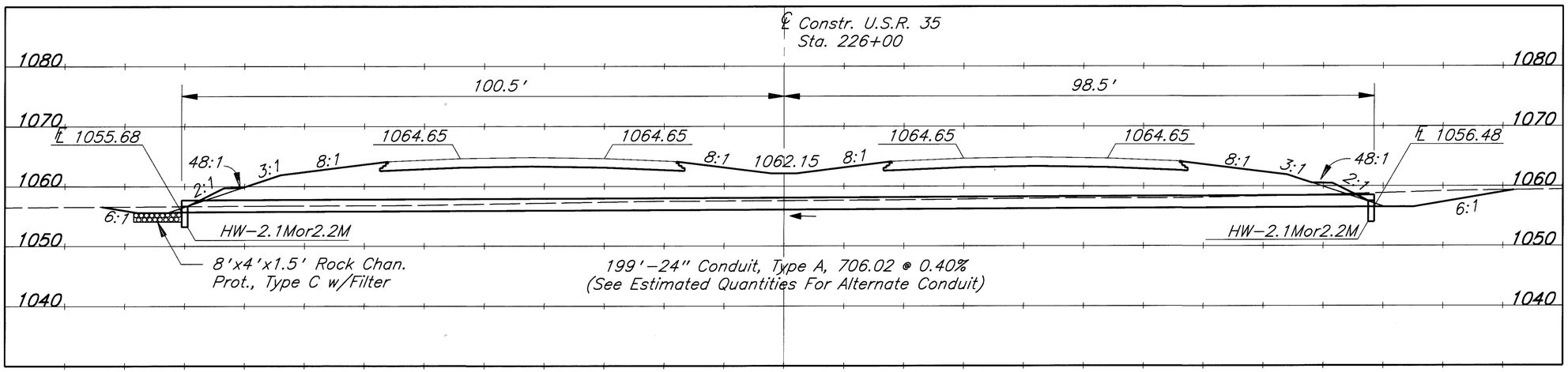
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=2.6 Ac.
Q_{50}	= 9 cfs
Q_{100}	=10 cfs
HW_{50}	= 1058.5
HW_{100}	= 1059.0
V_{50}	=5.2 fps
V_{100}	=5.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 0.9 Cu.Yd.
603	24" Conduit, Type A, 706.02, 707.01(.109), 707.01 Aluminum Coated(.064), 707.05(.079), or 707.21(.105)	=199 Lin.Ft.

* Carried to Culvert Subsummary Sheet



S-6A

HORIZONTAL SCALE IN FEET 0 5 10 20

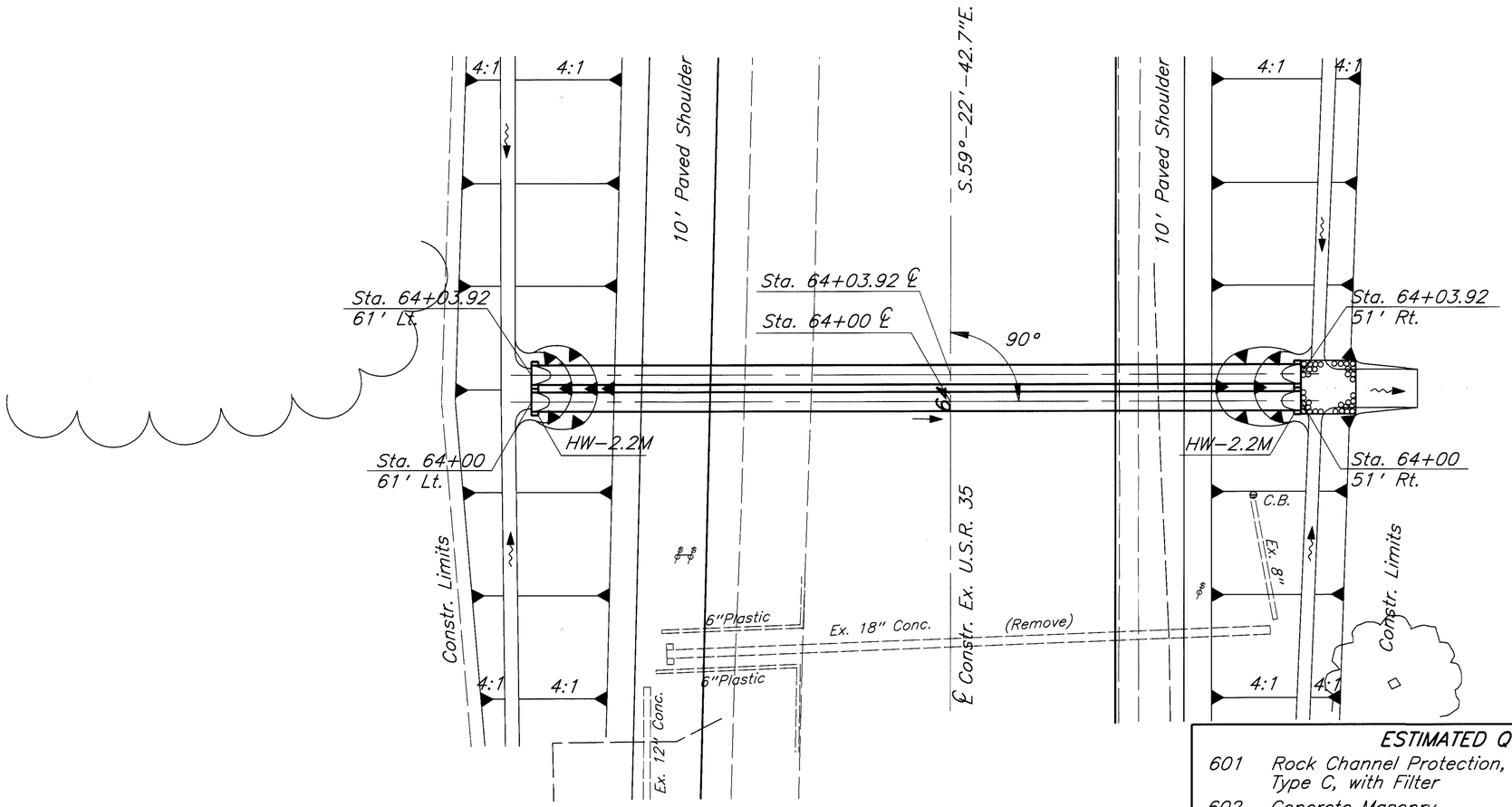
REVISED	CALCULATED	CHECKED
10-99	BY FYC	

CULVERT DETAIL S-6A
U.S.R. 35 – STA. 226 + 00

FAY-35-2.57

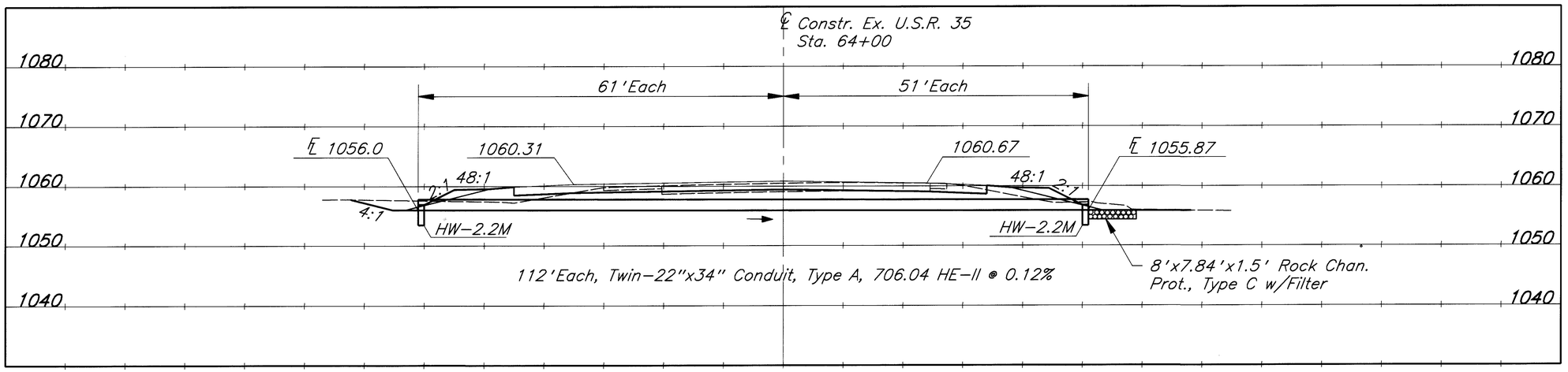
293
522

HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=45.5 Ac.
Q_{25}	=18 cfs Each
Q_{100}	=22 cfs Each
HW_{25}	= 1058.0
HW_{100}	= 1058.4
V_{25}	=6.0 fps
V_{100}	=6.6 fps



ESTIMATED QUANTITIES *		
601	Rock Channel Protection, Type C, with Filter	= 5 Cu.Yd.
602	Concrete Masonry	= 1.6 Cu.Yd.
603	22"x34" Conduit, Type A, 706.04 HE-II	=224 Lin.Ft.

* Carried to Culvert Subsummary Sheet



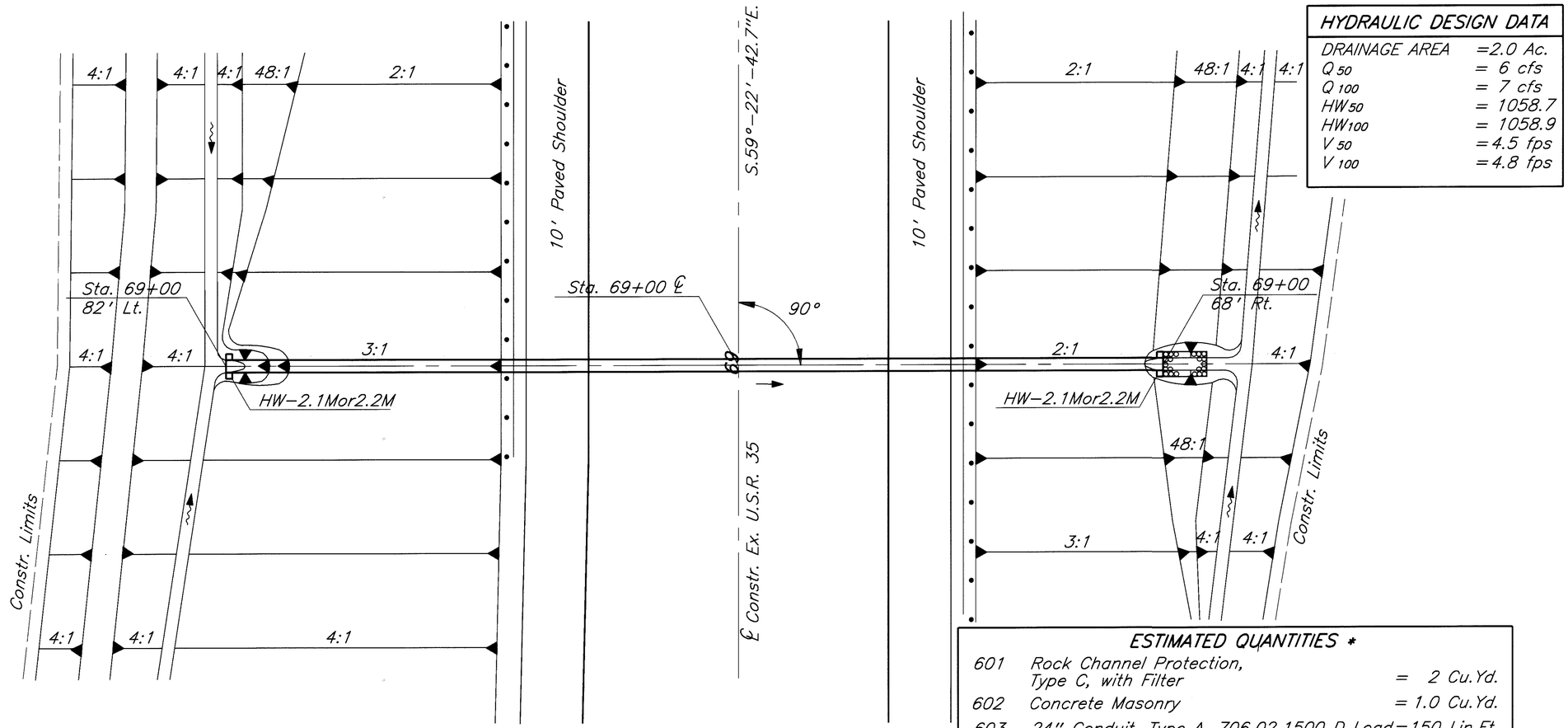
HORIZONTAL SCALE IN FEET: 0, 5, 10, 20

CALCULATED

CHECKED

CULVERT DETAIL S-7
EXIST. U.S.R. 35 STA. 64 + 00

FAY-35-2.57



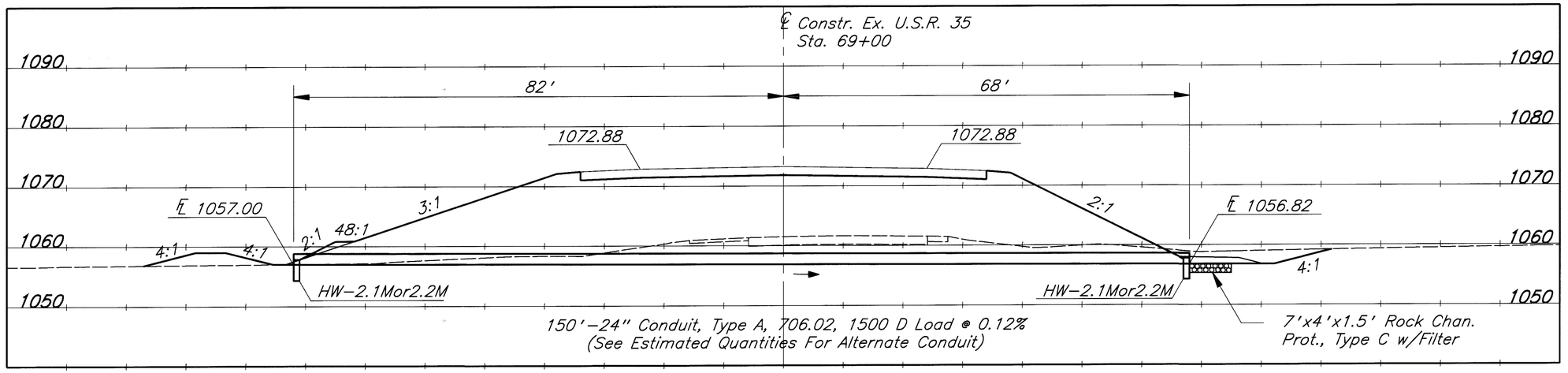
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=2.0 Ac.
Q 50	= 6 cfs
Q 100	= 7 cfs
HW50	= 1058.7
HW100	= 1058.9
V 50	=4.5 fps
V 100	=4.8 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 1.0 Cu.Yd.
603	24" Conduit, Type A, 706.02,1500 D Load or 707.01(.079), 707.01 Aluminum Coated (.064), 707.05, or 707.21(.060)	=150 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CALCULATED

CHECKED

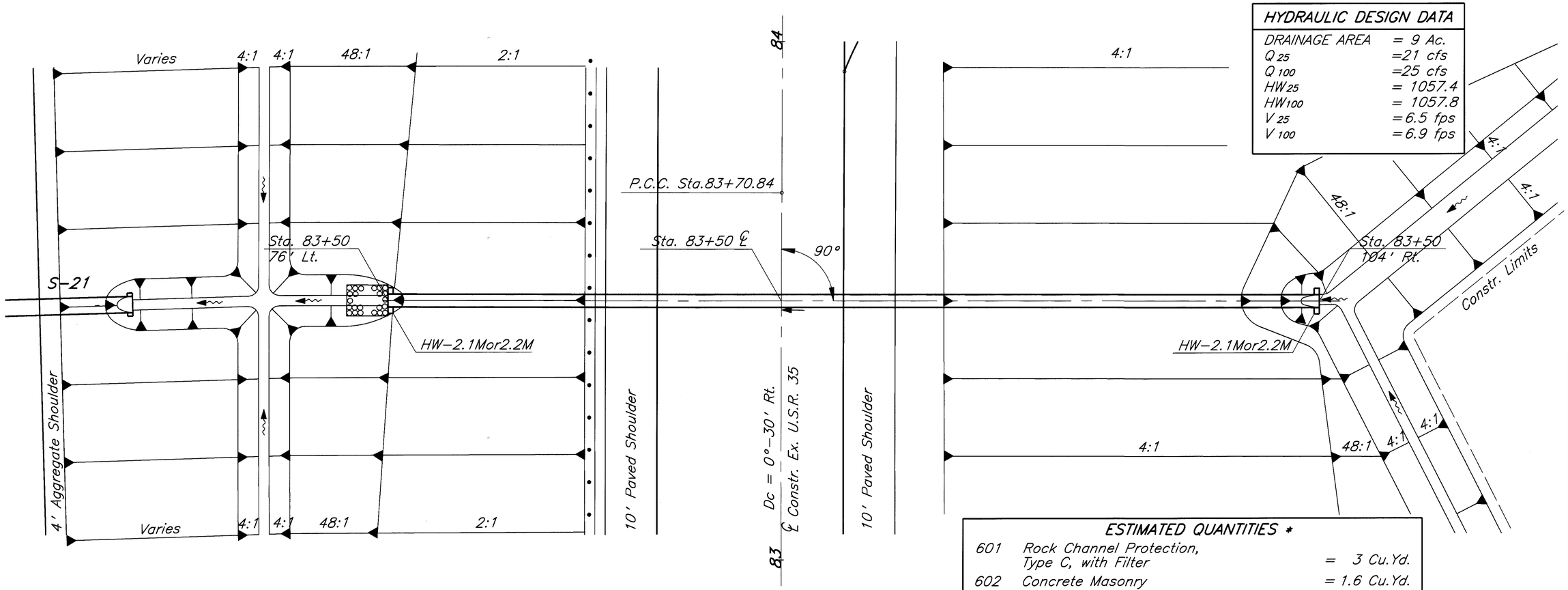
0 5 10 20 HORIZONTAL SCALE IN FEET

CULVERT DETAIL S-7A
EXIST. U.S.R. 35 STA. 69 + 00

FAY-35-2.57

S-7A

295
522



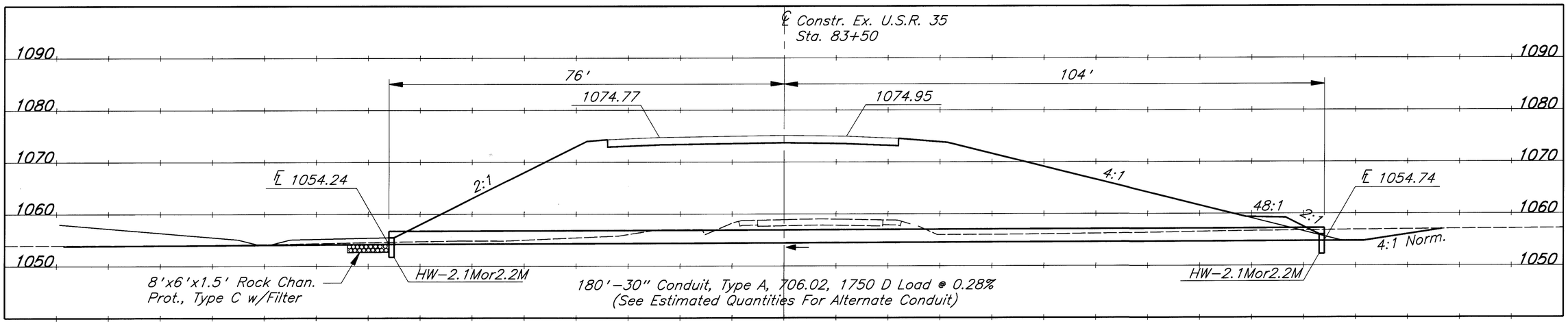
HYDRAULIC DESIGN DATA

DRAINAGE AREA	= 9 Ac.
Q_{25}	=21 cfs
Q_{100}	=25 cfs
HW_{25}	= 1057.4
HW_{100}	= 1057.8
V_{25}	=6.5 fps
V_{100}	=6.9 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 1.6 Cu.Yd.
603	30" Conduit, Type A, 706.02, 1750 D Load or 36" Conduit, Type A, 707.01(.109), 707.01 Aluminum Coated (.064), 707.05(.079), or 707.21(.105)	= 180 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CALCULATED

CHECKED

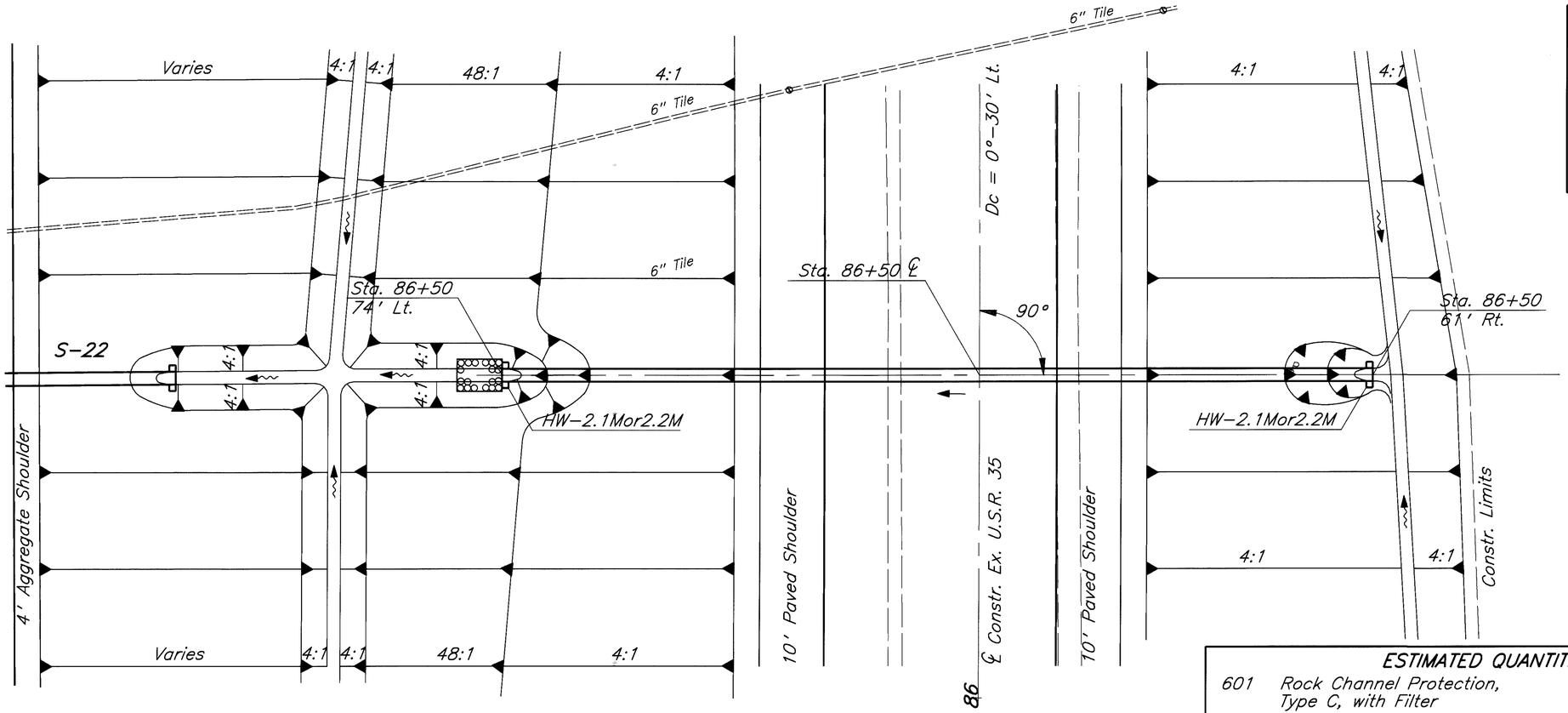
HORIZONTAL SCALE IN FEET 0 5 10 20

CULVERT DETAIL S-8
EXIST. U.S.R. 35 STA. 83 + 50

FAY-35-2.57

S-8

296
522

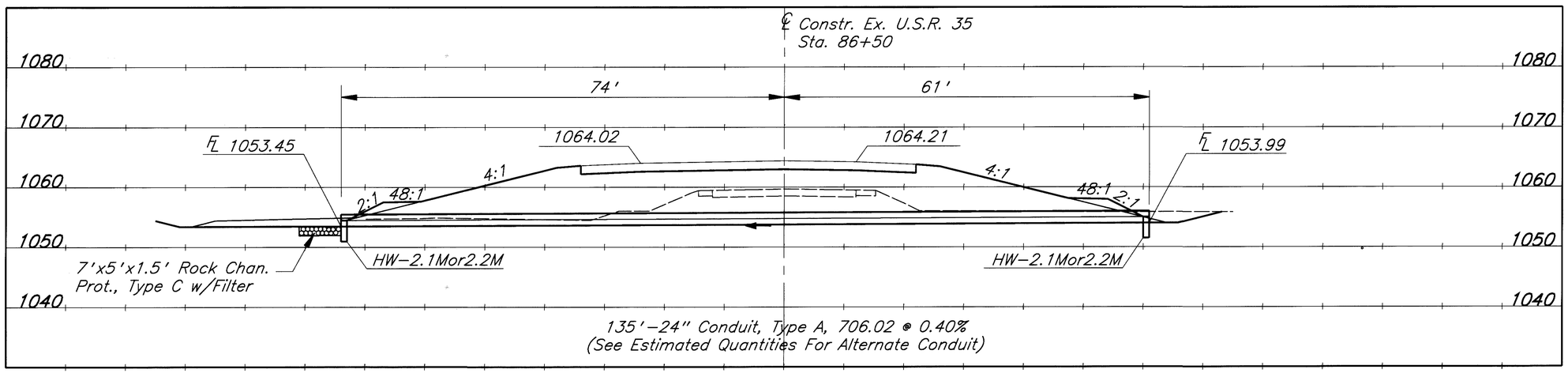


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	= 4 Ac.
Q_{25}	= 9 cfs
Q_{100}	=11 cfs
HW_{25}	= 1056.0
HW_{100}	= 1056.6
V_{25}	=4.2 fps
V_{100}	=5.1 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	24" Conduit, Type A, 706.02, 707.01(.079),707.01 Aluminum Coated (.064),707.05, or 707.21(.060)	=122 Lin.Ft.

* Carried to Culvert Subsummary Sheet



HORIZONTAL SCALE IN FEET
0 5 10 20

CALCULATED

CHECKED

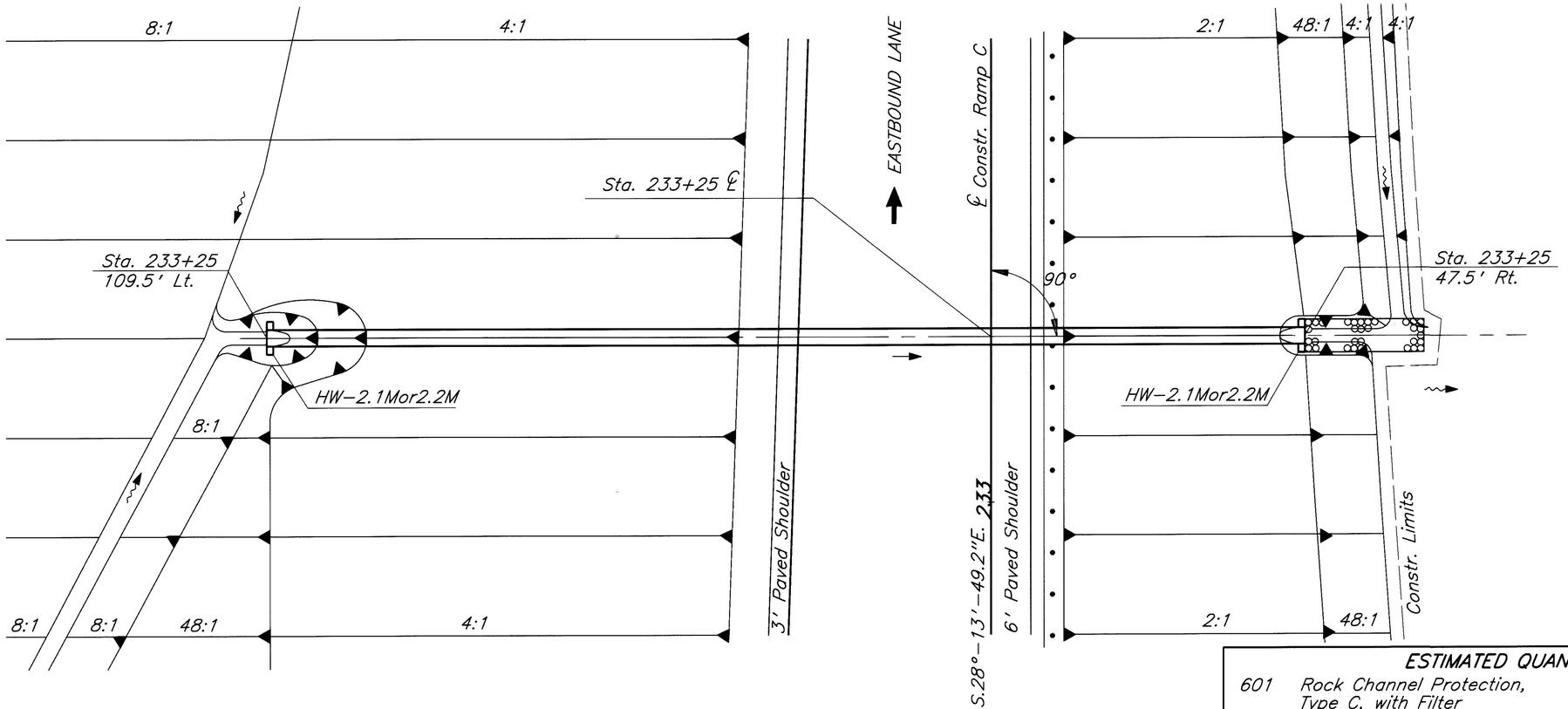
CULVERT DETAIL S-9
EXIST. U.S.R. 35 STA. 86 + 50

FAY-35-2.57

S-9

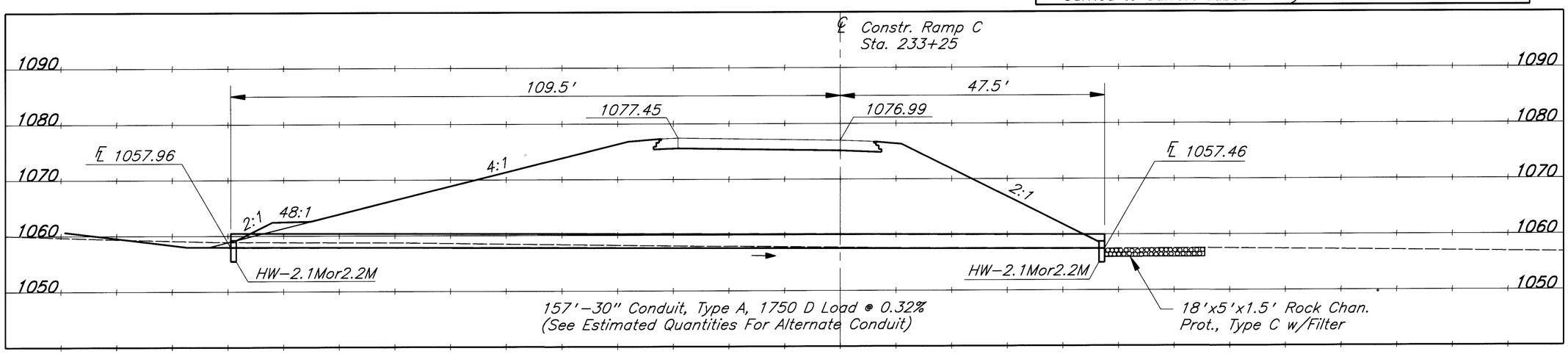
297
522

HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=2.7 Ac.
Q 25	= 8 cfs
Q 100	=10 cfs
HW25	= 1059.6
HW100	= 1059.8
V 25	=4.7 fps
V 100	=5.0 fps



ESTIMATED QUANTITIES *		
601	Rock Channel Protection, Type C, with Filter	= 5 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	30" Conduit, Type A, 706.02 1750 D Load or 707.01(.109), 707.01 Aluminum Coated (.064), 707.05(.079), or 707.21(.075)	=157 Lin.Ft.

* Carried to Culvert Subsummary Sheet



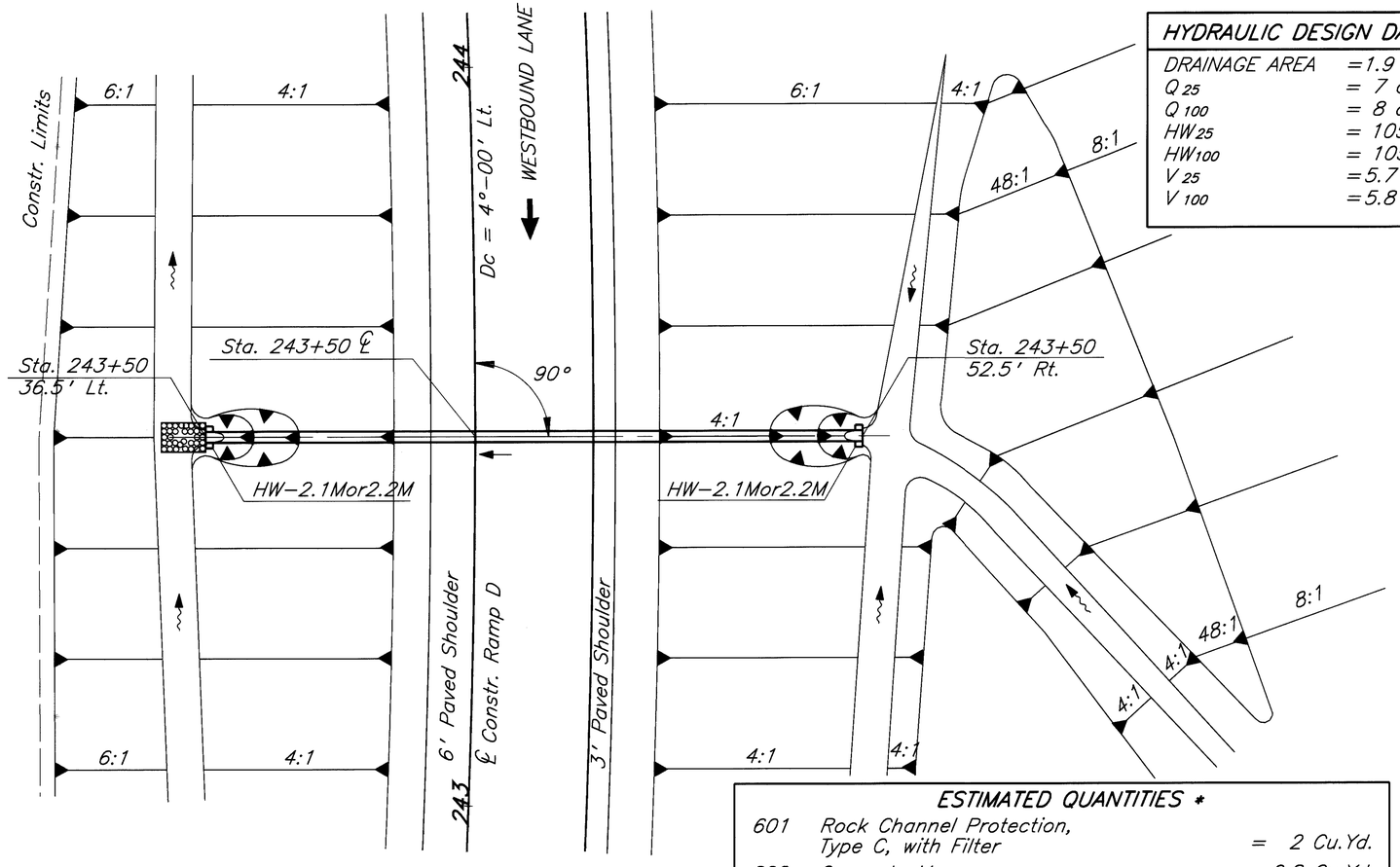
HORIZONTAL SCALE IN FEET 0 5 10 20

CALCULATED

CHECKED

CULVERT DETAIL S–10
RAMP C – STA. 233 + 25

FAY–35–2.57

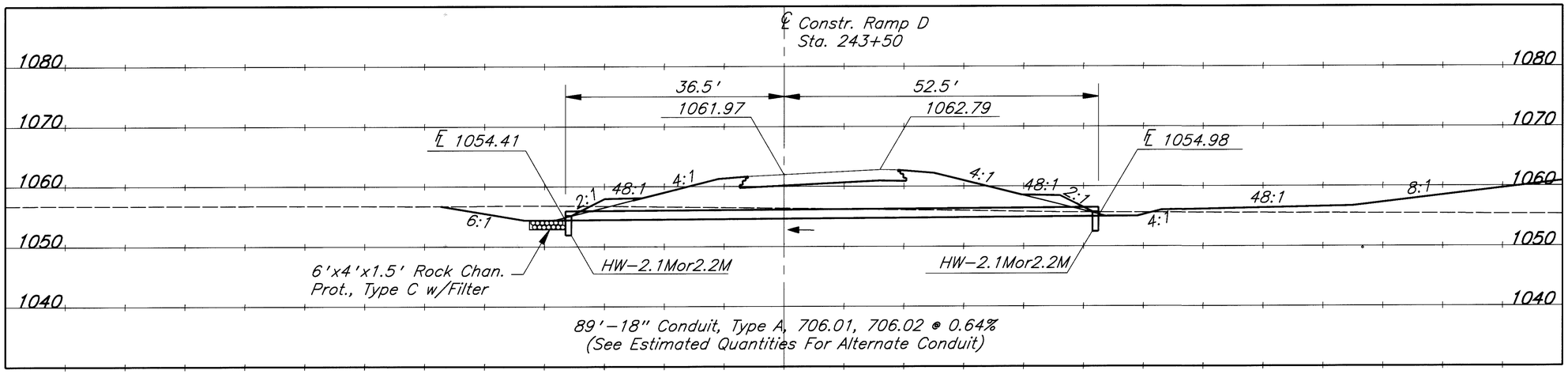


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=1.9 Ac.
Q_{25}	= 7 cfs
Q_{100}	= 8 cfs
HW_{25}	= 1056.7
HW_{100}	= 1056.9
V_{25}	=5.7 fps
V_{100}	=5.8 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 0.8 Cu.Yd.
603	18" Conduit, Type A, 706.01, 706.02, or 21" Conduit, Type A 707.01, or 707.21	= 89 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CALCULATED | CHECKED

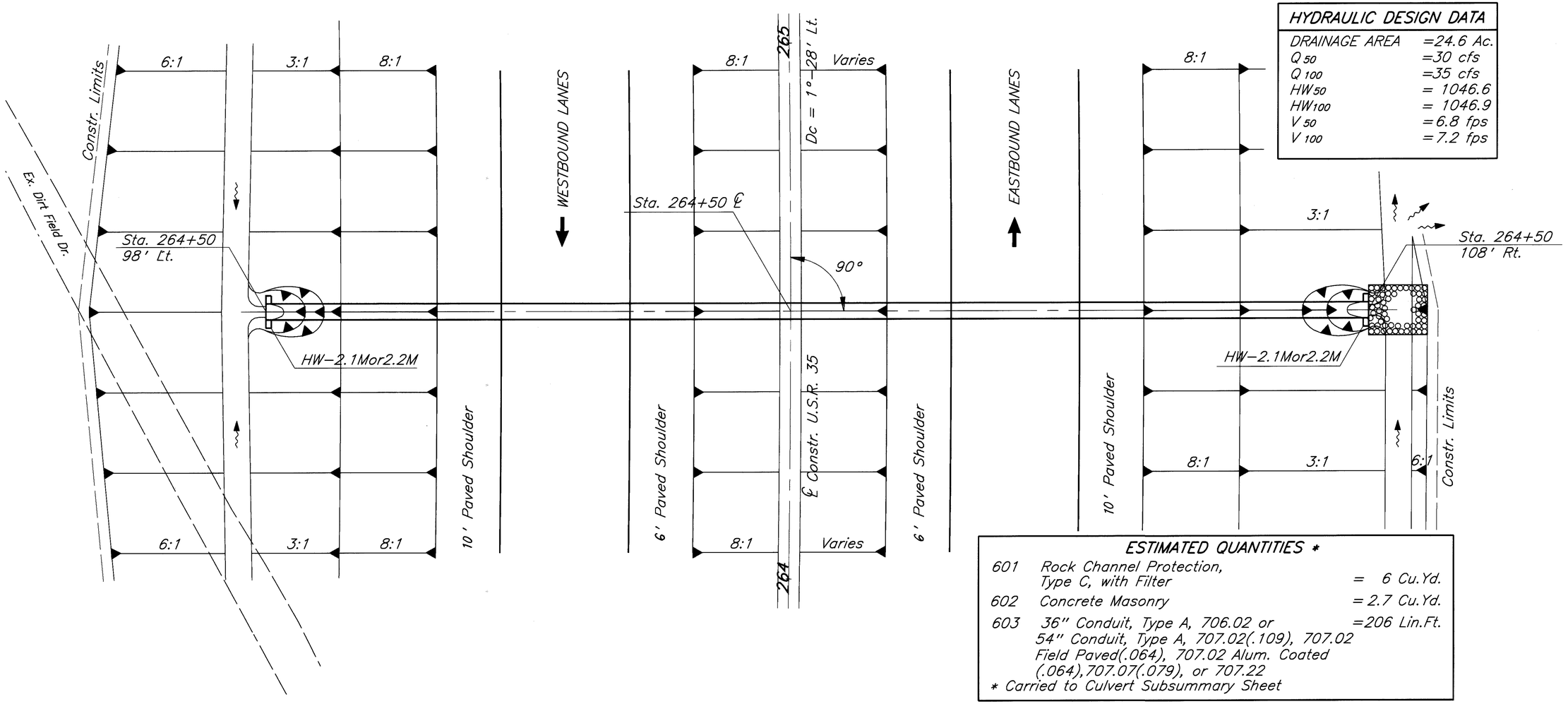
HORIZONTAL SCALE IN FEET: 0, 5, 10, 20

CULVERT DETAIL S–11
RAMP D – STA. 243 + 50

FAY–35–2.57

S–11

299
522



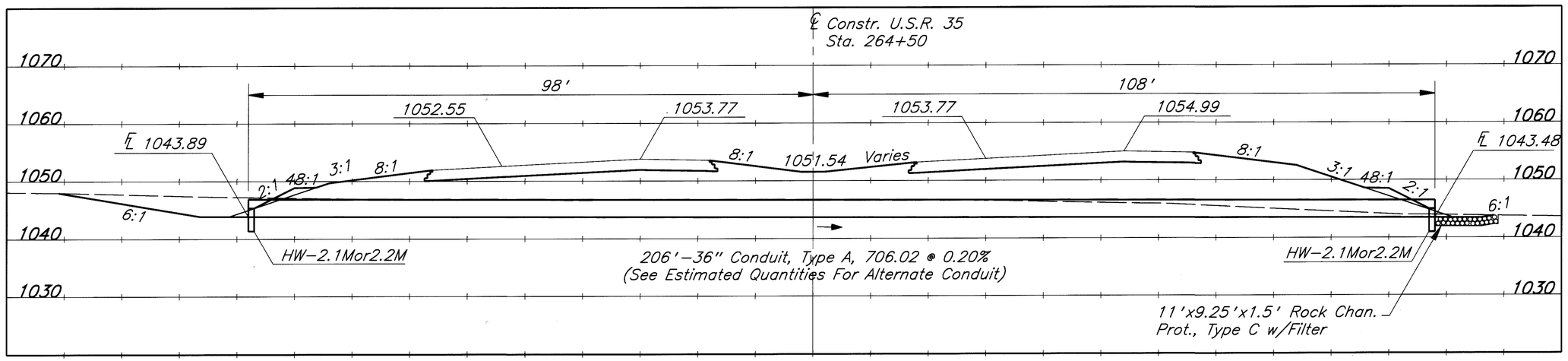
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=24.6 Ac.
Q_{50}	=30 cfs
Q_{100}	=35 cfs
HW_{50}	= 1046.6
HW_{100}	= 1046.9
V_{50}	=6.8 fps
V_{100}	=7.2 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 6 Cu.Yd.
602	Concrete Masonry	= 2.7 Cu.Yd.
603	36" Conduit, Type A, 706.02 or 54" Conduit, Type A, 707.02(.109), 707.02 Field Paved(.064), 707.02 Alum. Coated (.064),707.07(.079), or 707.22	=206 Lin.Ft.

* Carried to Culvert Subsummary Sheet



0 5 10 20 HORIZONTAL SCALE IN FEET

REVISED	CALCULATED	CHECKED
10-99		
BY		
FYC		

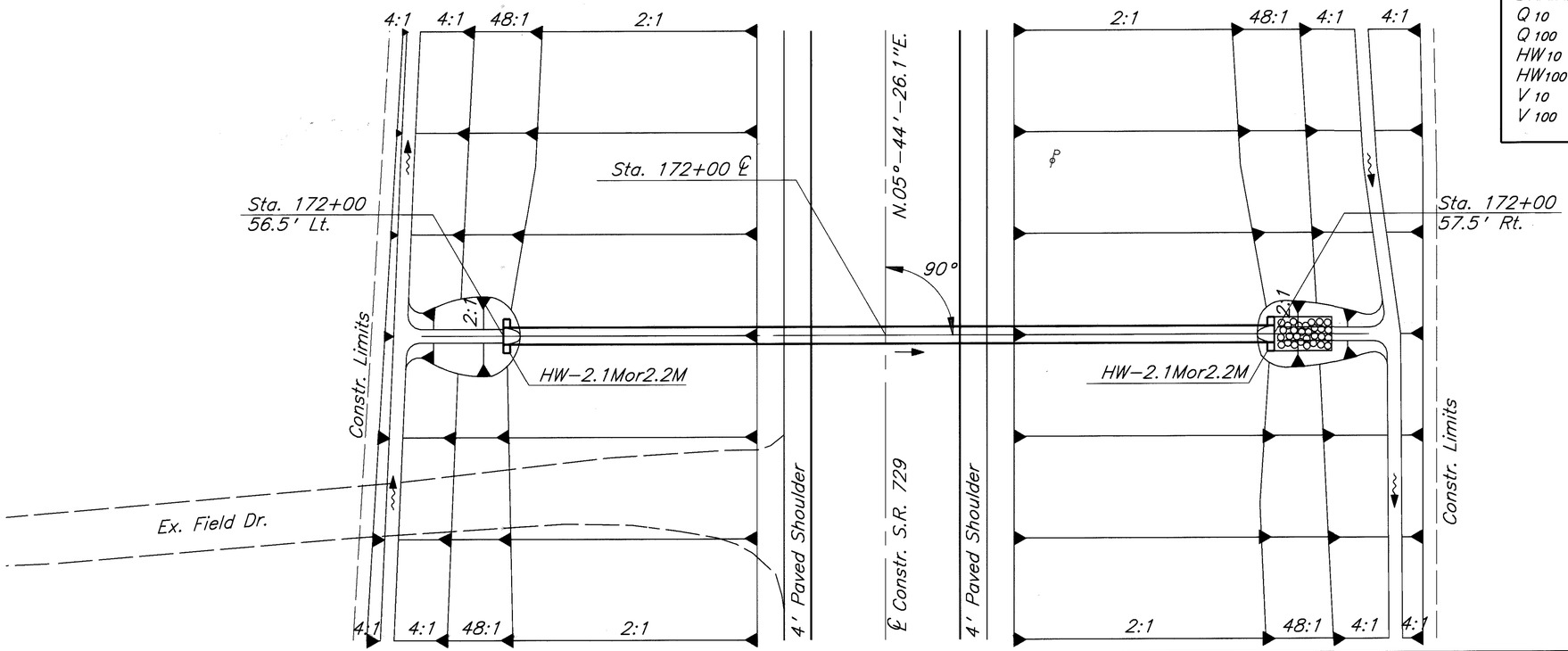
CULVERT DETAIL S-12
U.S.R. 35 - STA. 264 + 50

FAY-35-2.57

S-12

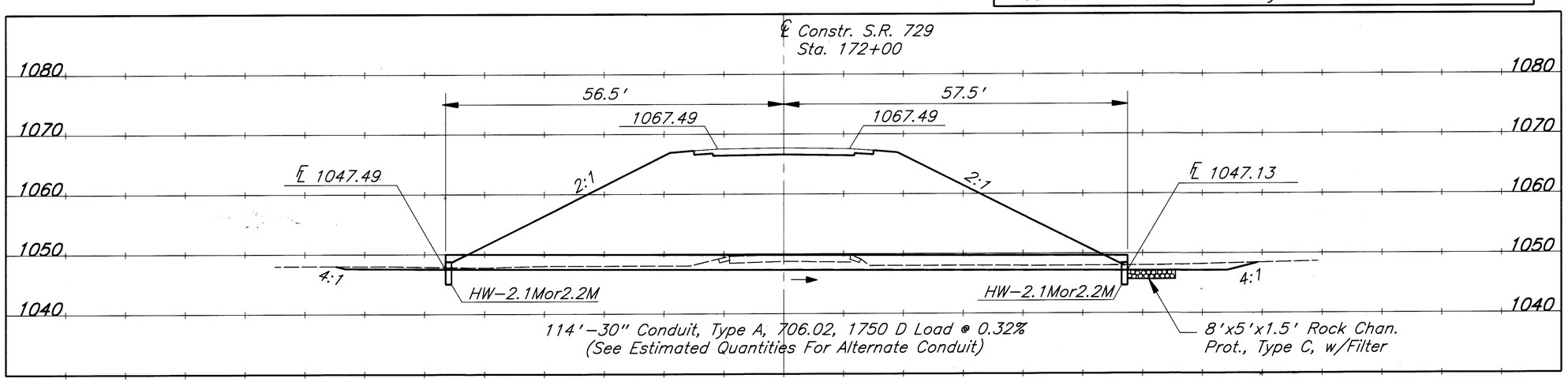
300
522

HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=8.5 Ac.
Q_{10}	=14 cfs
Q_{100}	=20 cfs
HW_{10}	= 1049.4
HW_{100}	= 1049.8
V_{10}	=5.6 fps
V_{100}	=6.4 fps



ESTIMATED QUANTITIES *		
601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	30" Conduit, Type A, 706.02, 1750 D Load or 42" Conduit, Type A 707.01 Aluminum Coated(.109),707.05, or 707.21(.105)	= 114 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CULVERT DETAIL S-13
S.R. 729 - STA. 172 + 00

CALCULATED

CHECKED

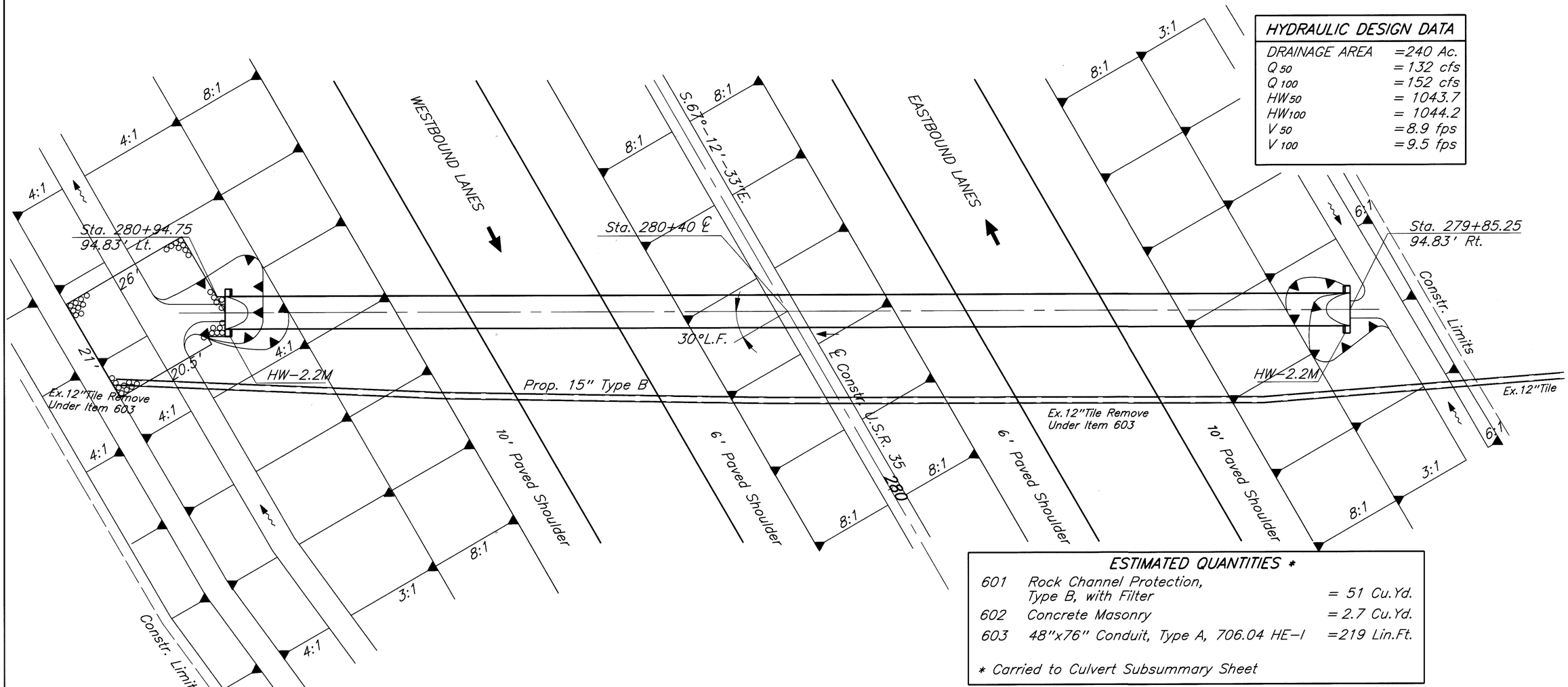
HORIZONTAL SCALE IN FEET
0 5 10 20

FAY-35-2.57

S-13

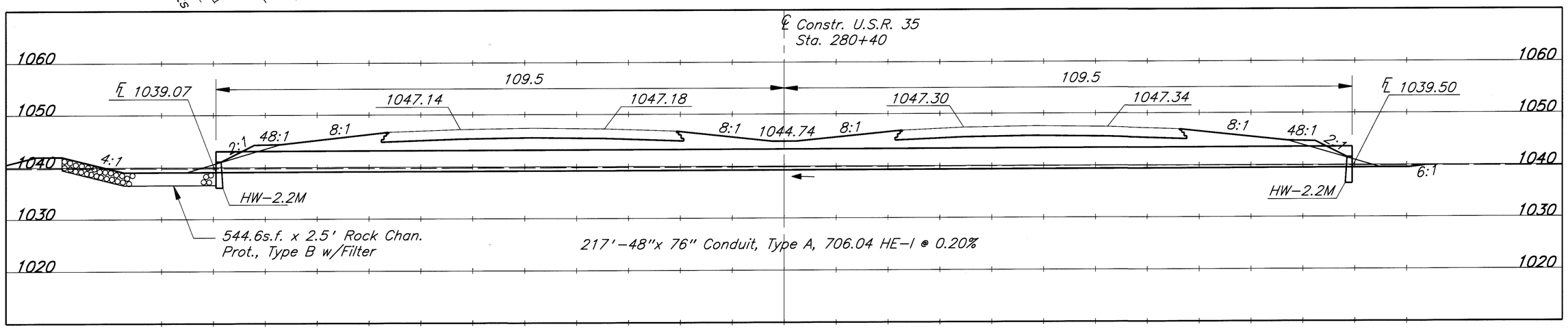
301
522

HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=240 Ac.
Q 50	= 132 cfs
Q 100	= 152 cfs
HW 50	= 1043.7
HW 100	= 1044.2
V 50	=8.9 fps
V 100	=9.5 fps



ESTIMATED QUANTITIES *		
601	Rock Channel Protection, Type B, with Filter	= 51 Cu.Yd.
602	Concrete Masonry	= 2.7 Cu.Yd.
603	48"x76" Conduit, Type A, 706.04 HE-I	=219 Lin.Ft.

* Carried to Culvert Subsummary Sheet



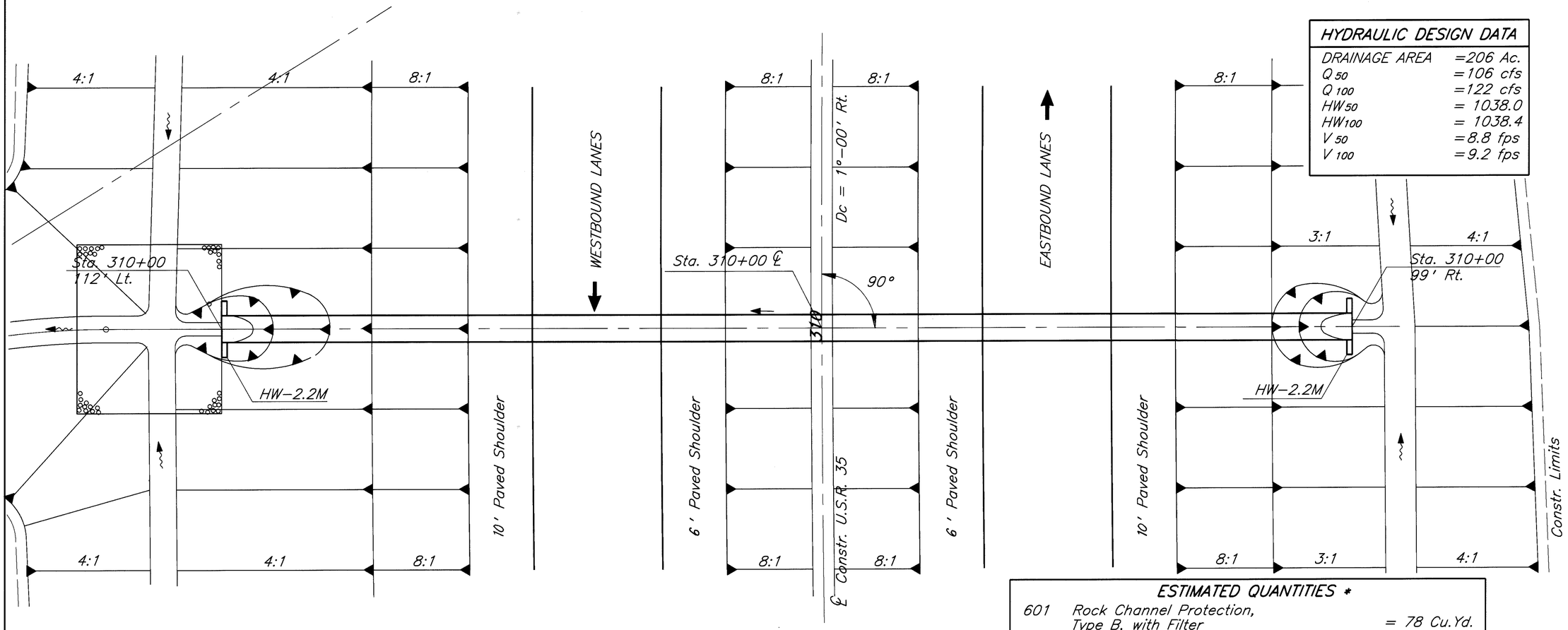
S-14

REVISED 10-99 BY FYC
CALCULATED
CHECKED
HORIZONTAL SCALE IN FEET 0 5 10 20

CULVERT DETAIL S-14
U.S.R. 35 - STA. 280 + 40

FAY-35-2.57

302
522

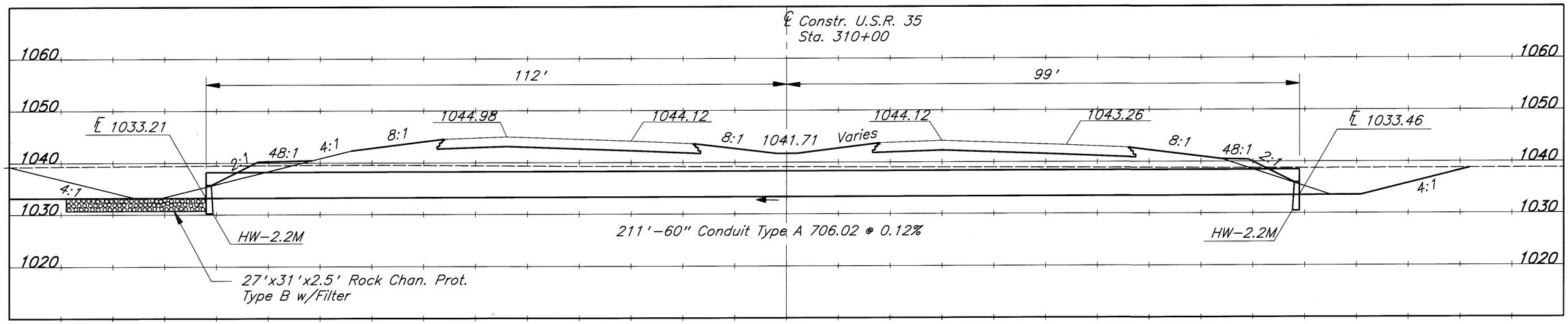


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=206 Ac.
Q_{50}	=106 cfs
Q_{100}	=122 cfs
HW_{50}	= 1038.0
HW_{100}	= 1038.4
V_{50}	=8.8 fps
V_{100}	=9.2 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type B, with Filter	= 78 Cu.Yd.
602	Concrete Masonry	= 3.9 Cu.Yd.
603	60" Conduit, Type A, 706.02	=211 Lin.Ft.

* Carried to Culvert Subsummary Sheet



S-15

HORIZONTAL SCALE IN FEET 0 5 10 20

CALCULATED

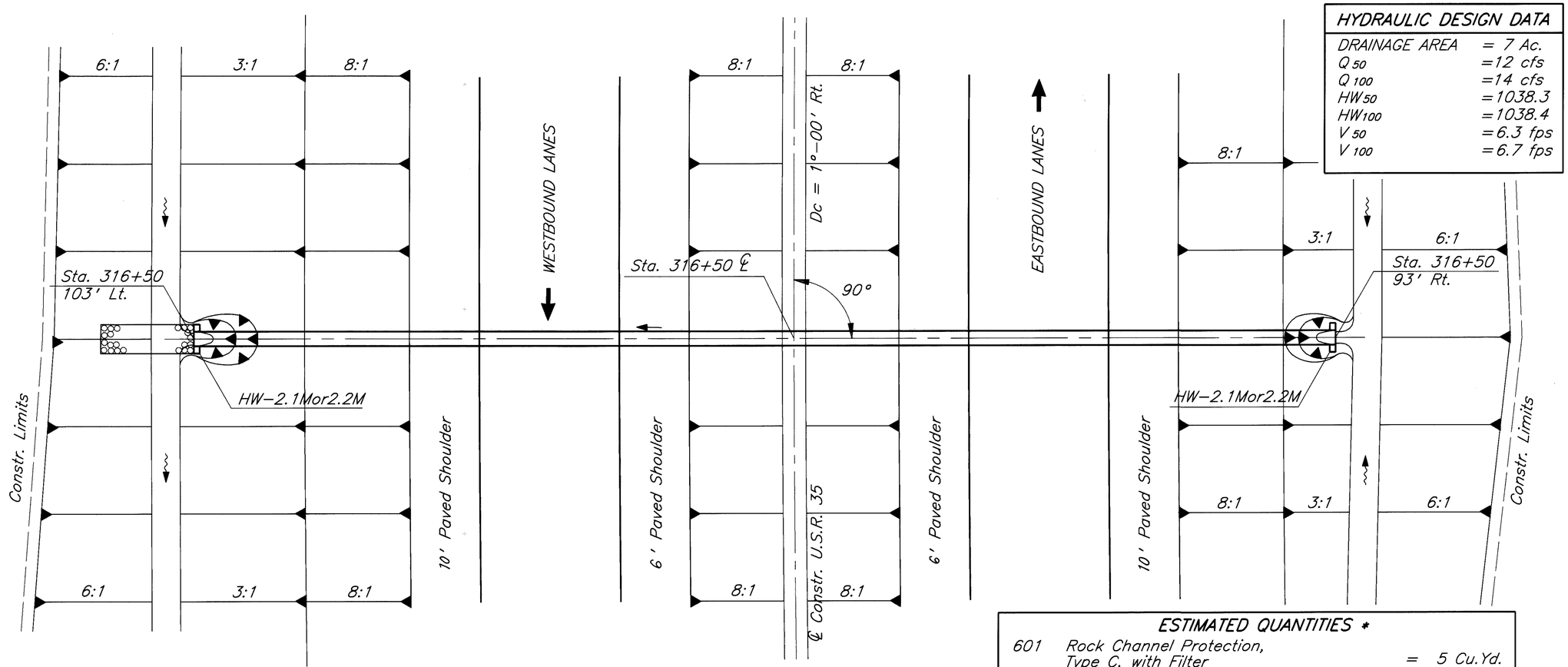
CHECKED

REVISED 10-99 BY FYC

CULVERT DETAIL S-15
U.S.R. 35 – STA. 310 + 00

FAY-35-2.57

303
522



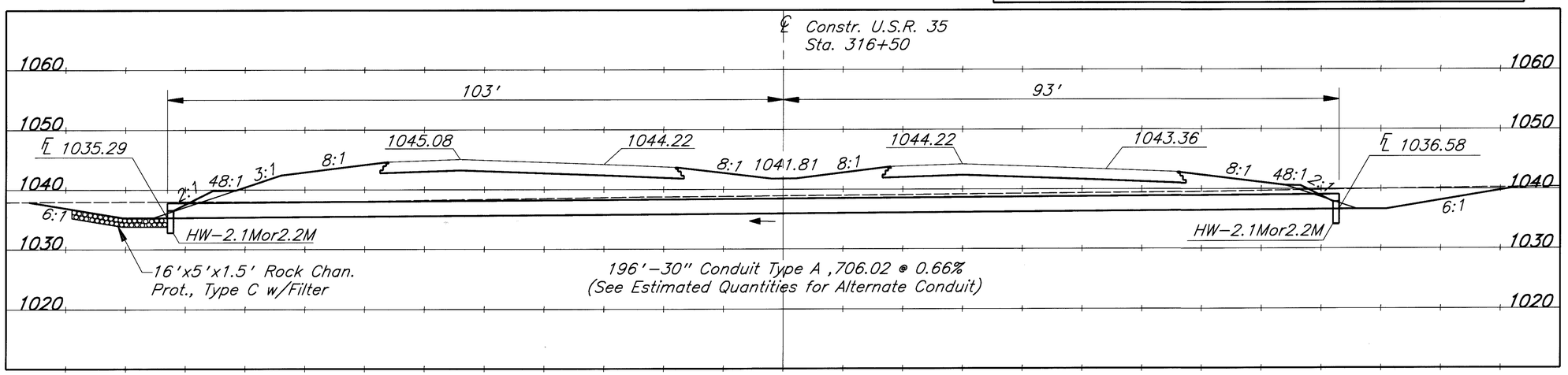
HYDRAULIC DESIGN DATA

DRAINAGE AREA	= 7 Ac.
Q_{50}	=12 cfs
Q_{100}	=14 cfs
HW_{50}	=1038.3
HW_{100}	=1038.4
V_{50}	=6.3 fps
V_{100}	=6.7 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 5 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	30" Conduit, Type A, 706.02,707.01(.109),=196 Lin.Ft. 707.01 Aluminum Coated(.064),707.05(.079), or 707.21(.075)	

* Carried to Culvert Subsummary Sheet



REVISED 10-99 BY FYC | CALCULATED | CHECKED

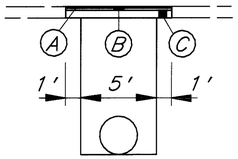
HORIZONTAL SCALE IN FEET 0 5 10 20

CULVERT DETAIL S–16
U.S.R. 35 – STA. 316 + 50

FAY–35–2.57

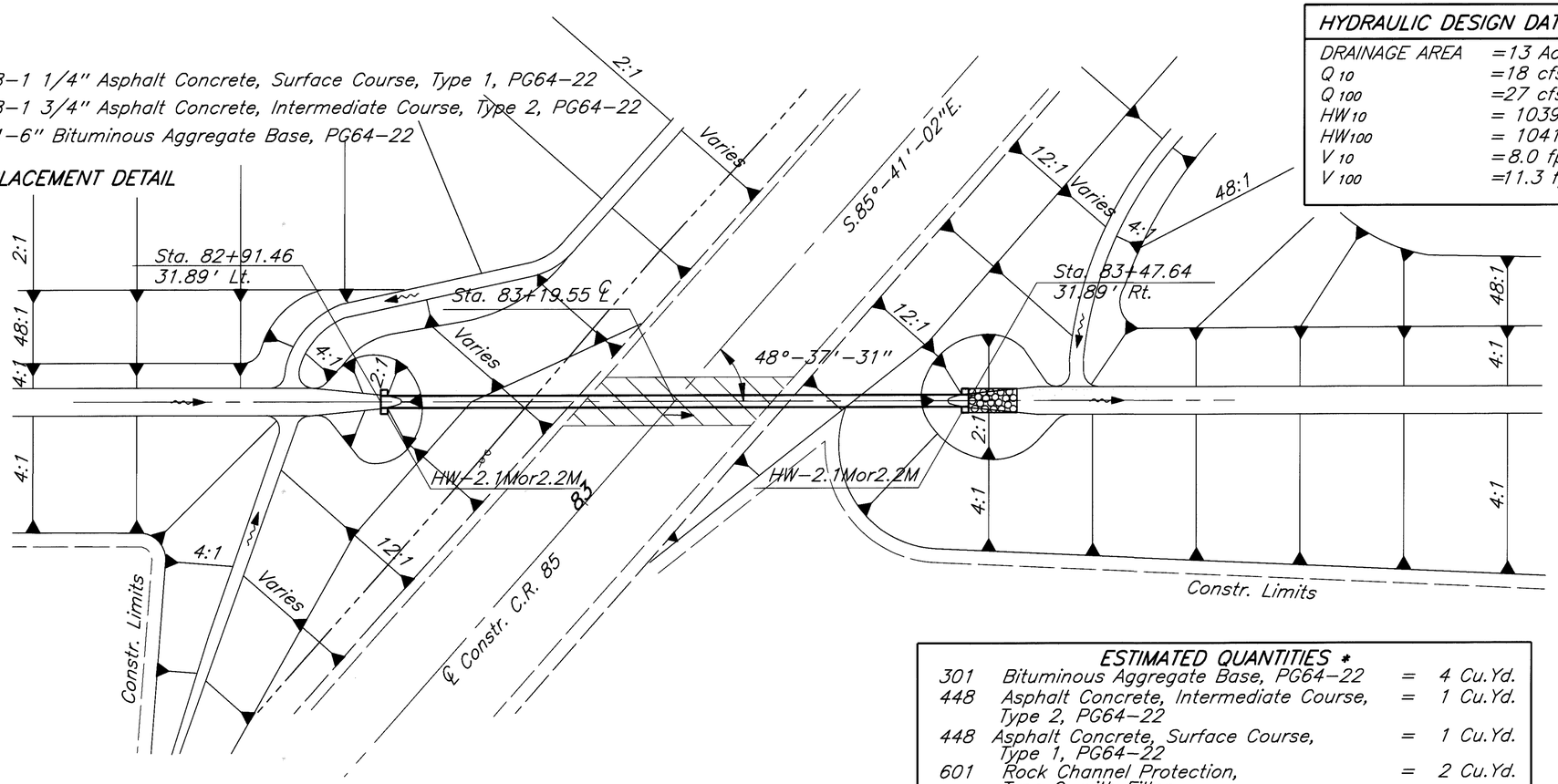
S–16

304
522



(A) 448–1 1/4" Asphalt Concrete, Surface Course, Type 1, PG64–22
(B) 448–1 3/4" Asphalt Concrete, Intermediate Course, Type 2, PG64–22
(C) 301–6" Bituminous Aggregate Base, PG64–22

PAVEMENT REPLACEMENT DETAIL

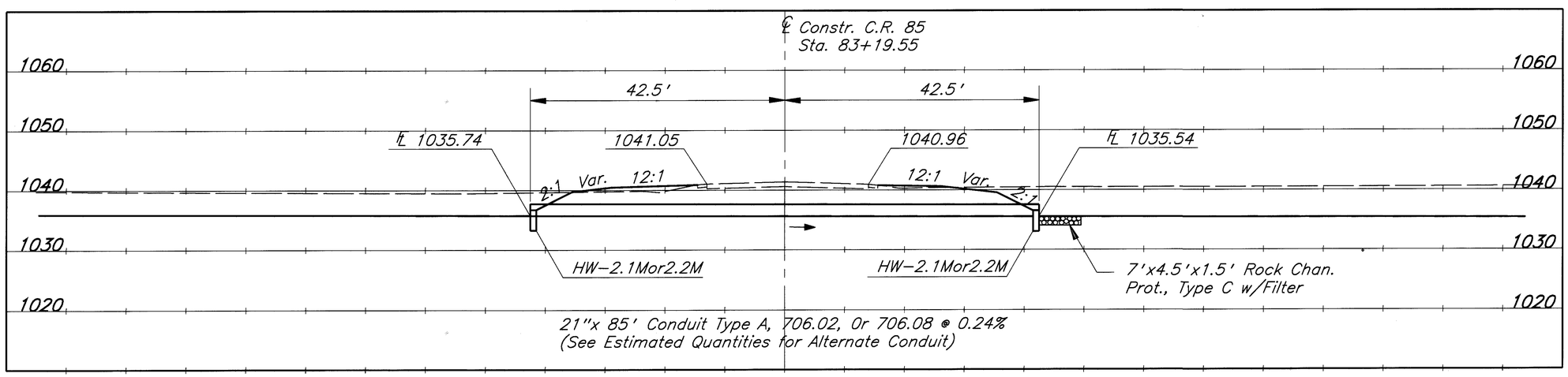


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=13 Ac.
Q 10	=18 cfs
Q 100	=27 cfs
HW 10	= 1039.2
HW 100	= 1041.9
V 10	=8.0 fps
V 100	=11.3 fps

ESTIMATED QUANTITIES *

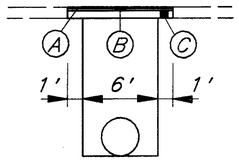
301	Bituminous Aggregate Base, PG64–22	= 4 Cu.Yd.
448	Asphalt Concrete, Intermediate Course, Type 2, PG64–22	= 1 Cu.Yd.
448	Asphalt Concrete, Surface Course, Type 1, PG64–22	= 1 Cu.Yd.
601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 1.1 Cu.Yd.
603	21" Conduit, Type A, 706.02 or 706.08 or 27" Conduit, Type A, 707.05	= 85 Lin.Ft.

* Carried to Culvert Subsummary Sheet



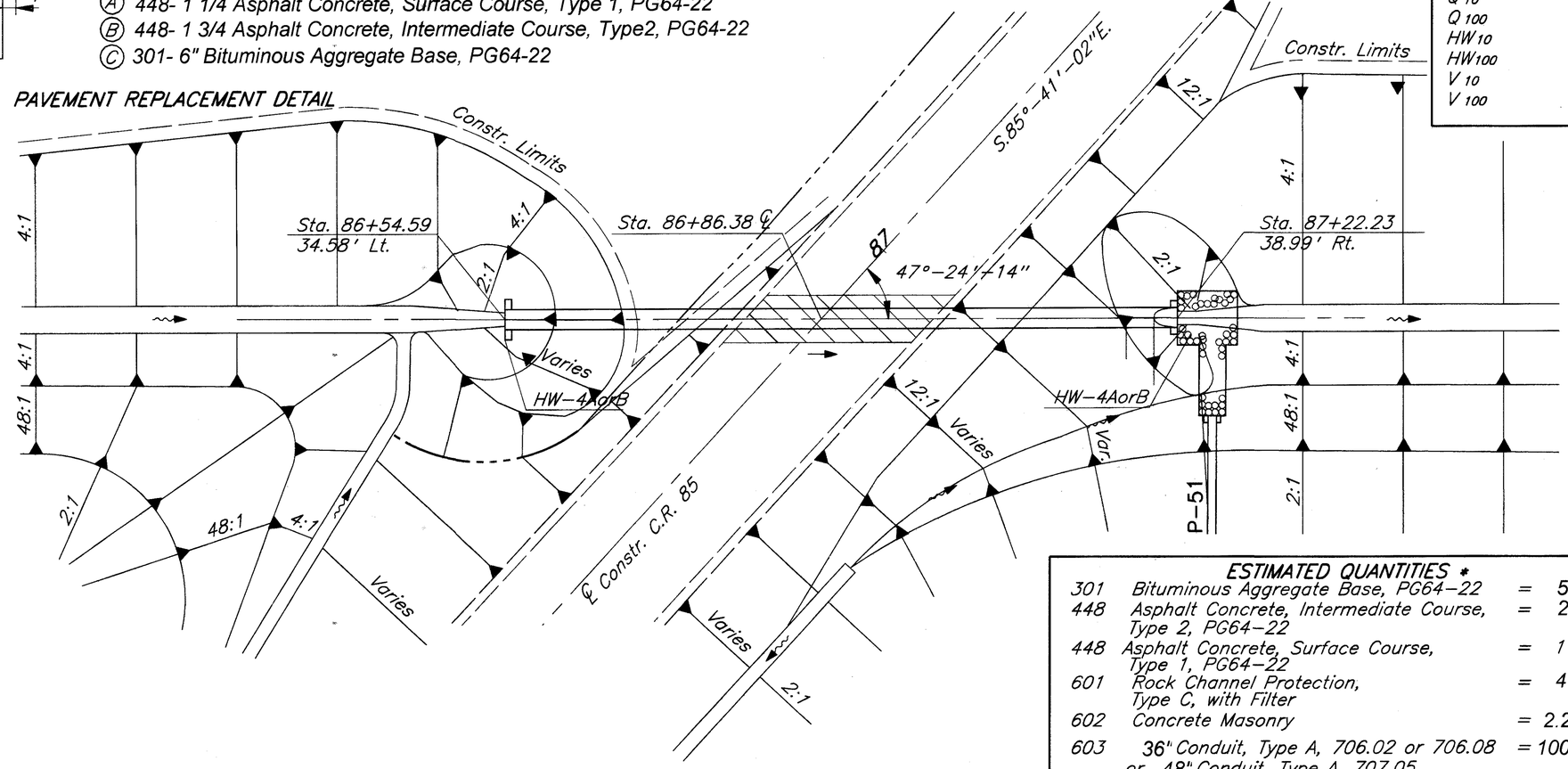
CULVERT DETAIL S–17
C.R. 85 – STA. 83 + 19.55

FAY–35–2.57



(A) 448- 1 1/4 Asphalt Concrete, Surface Course, Type 1, PG64-22
(B) 448- 1 3/4 Asphalt Concrete, Intermediate Course, Type2, PG64-22
(C) 301- 6" Bituminous Aggregate Base, PG64-22

PAVEMENT REPLACEMENT DETAIL



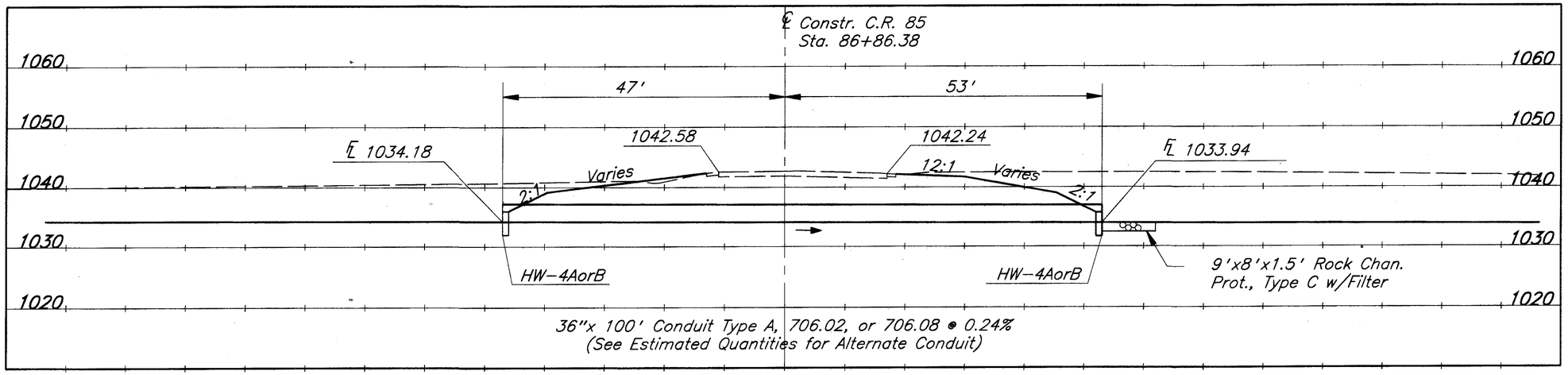
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=72 Ac.
Q 10	=51 cfs
Q 100	=77 cfs
HW 10	= 1038.1
HW 100	= 1040.4
V 10	= 8.6 fps
V 100	=11.3 fps

ESTIMATED QUANTITIES *

301	Bituminous Aggregate Base, PG64-22	=	5 Cu.Yd.
448	Asphalt Concrete, Intermediate Course, Type 2, PG64-22	=	2 Cu.Yd.
448	Asphalt Concrete, Surface Course, Type 1, PG64-22	=	1 Cu.Yd.
601	Rock Channel Protection, Type C, with Filter	=	4 Cu.Yd.
602	Concrete Masonry	=	2.2 Cu.Yd.
603	36" Conduit, Type A, 706.02 or 706.08 or 48" Conduit, Type A, 707.05	=	100 Lin.Ft.

* Carried to Culvert Subsummary Sheet



Revised 7/25/00, By: E.R.P.

S-18

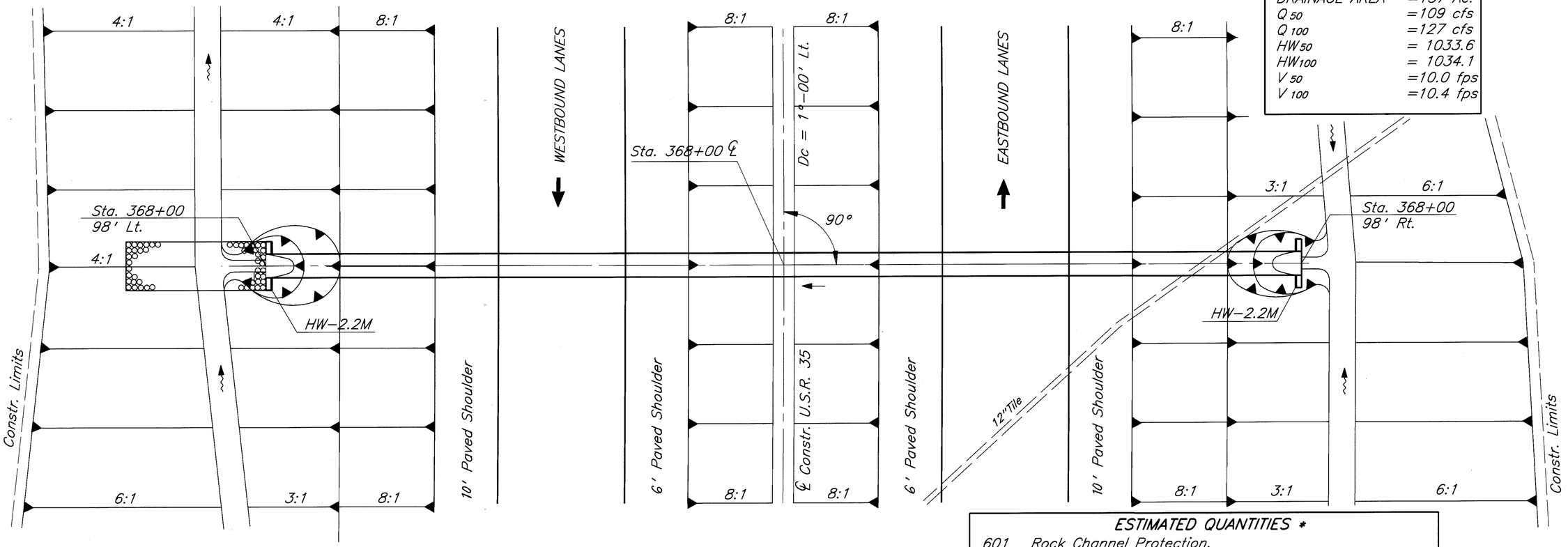
CULVERT DETAIL S-18
C.R. 85 - STA. 86 + 86.38

CALCULATED | CHECKED

HORIZONTAL SCALE IN FEET
0 5 10 20

FAY-35-2.57

306
522



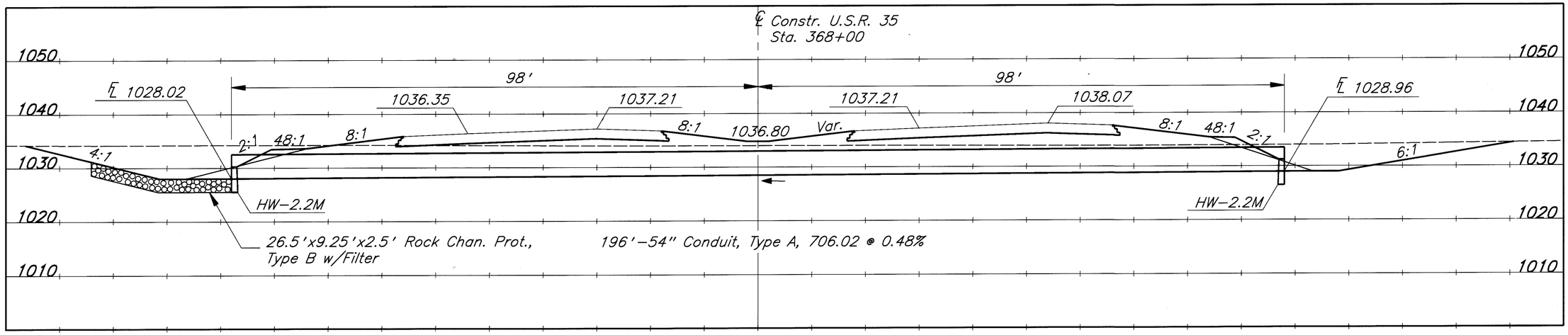
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=137 Ac.
Q 50	=109 cfs
Q 100	=127 cfs
HW 50	= 1033.6
HW 100	= 1034.1
V 50	=10.0 fps
V 100	=10.4 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type B, with Filter	= 23 Cu.Yd.
602	Concrete Masonry	= 2.7 Cu.Yd.
603	54" Conduit, Type A, 706.02	=196 Lin.Ft.

* Carried to Culvert Subsummary Sheet



S-19

0 5 10 20 HORIZONTAL SCALE IN FEET

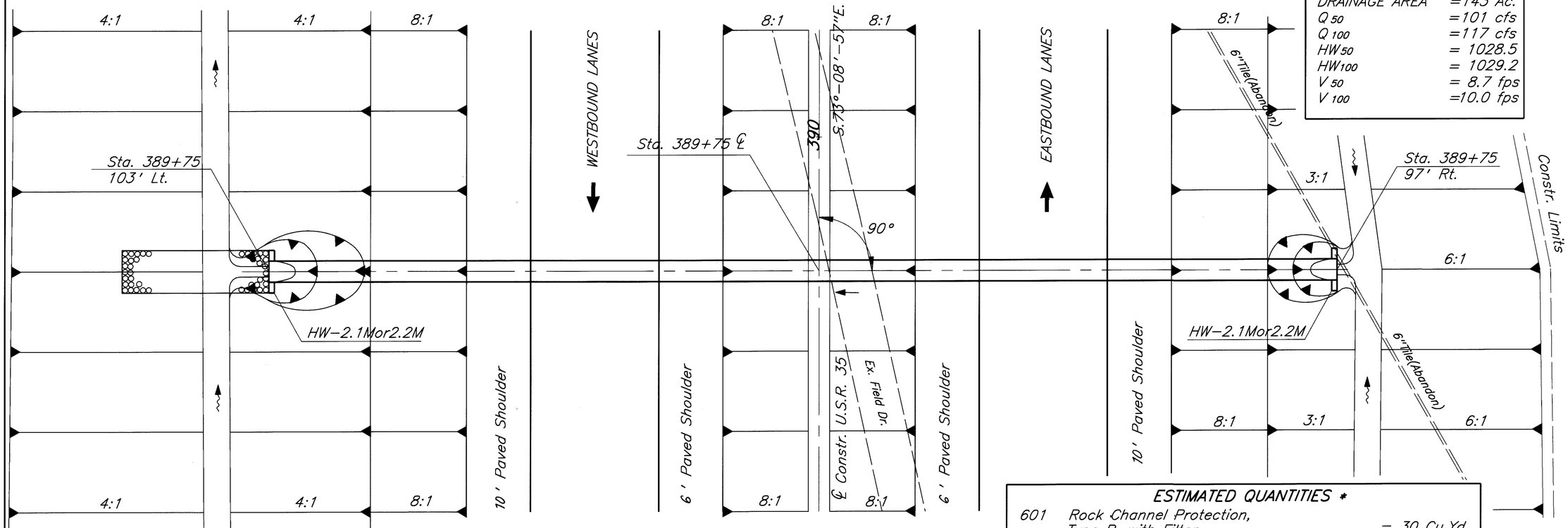
CALCULATED | CHECKED

REVISED 10-99 BY FYC

CULVERT DETAIL S-19
U.S.R. 35 - STA. 368 + 00

FAY-35-2.57

307
522



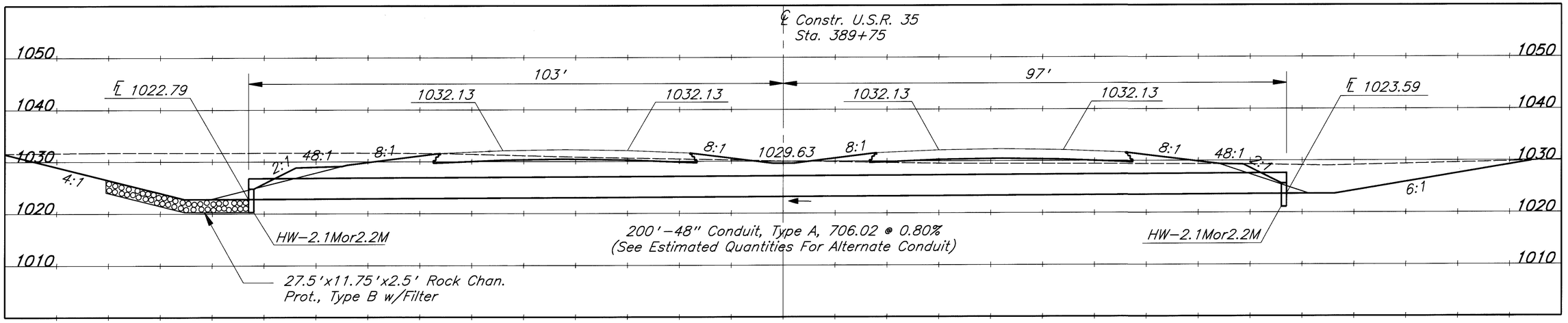
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=143 Ac.
Q 50	=101 cfs
Q 100	=117 cfs
HW50	= 1028.5
HW100	= 1029.2
V 50	= 8.7 fps
V 100	=10.0 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type B, with Filter	= 30 Cu.Yd.
602	Concrete Masonry	= 4.2 Cu.Yd.
603	48" Conduit, Type A, 706.02 or 66" Conduit, Type A, 707.02,Field Paved (.064),707.02 (.109), 707.02 Aluminum Coated (.079) 707.04, 707.07 (.079),or 707.22	=200 Lin.Ft.

* Carried to Culvert Subsummary Sheet



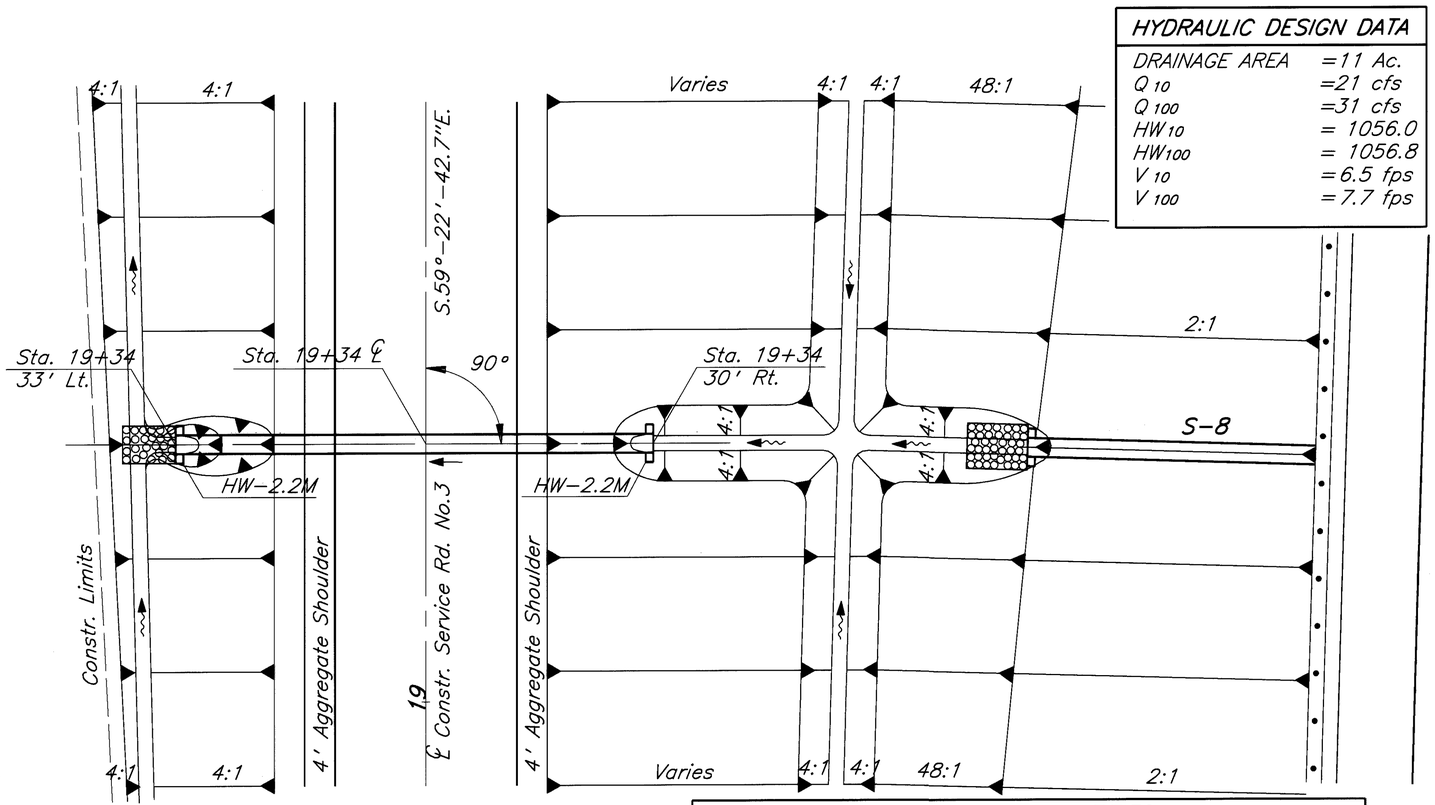
REVISED 10-99 BY FYC
CALCULATED
CHECKED
0 5 10 20 HORIZONTAL SCALE IN FEET

CULVERT DETAIL S-20
U.S.R. 35 – STA. 389 + 75

FAY-35-2.57

S-20

308
522



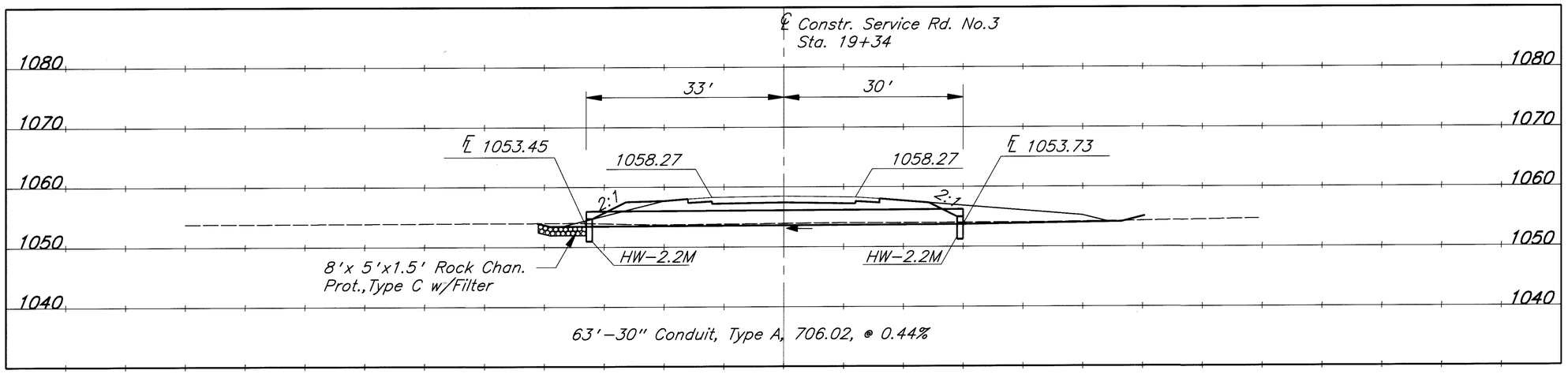
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=11 Ac.
Q_{10}	=21 cfs
Q_{100}	=31 cfs
HW_{10}	= 1056.0
HW_{100}	= 1056.8
V_{10}	=6.5 fps
V_{100}	=7.7 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 1.2 Cu.Yd.
603	30" Conduit, Type A, 706.02	= 63 Lin.Ft.

* Carried to Culvert Subsummary Sheet



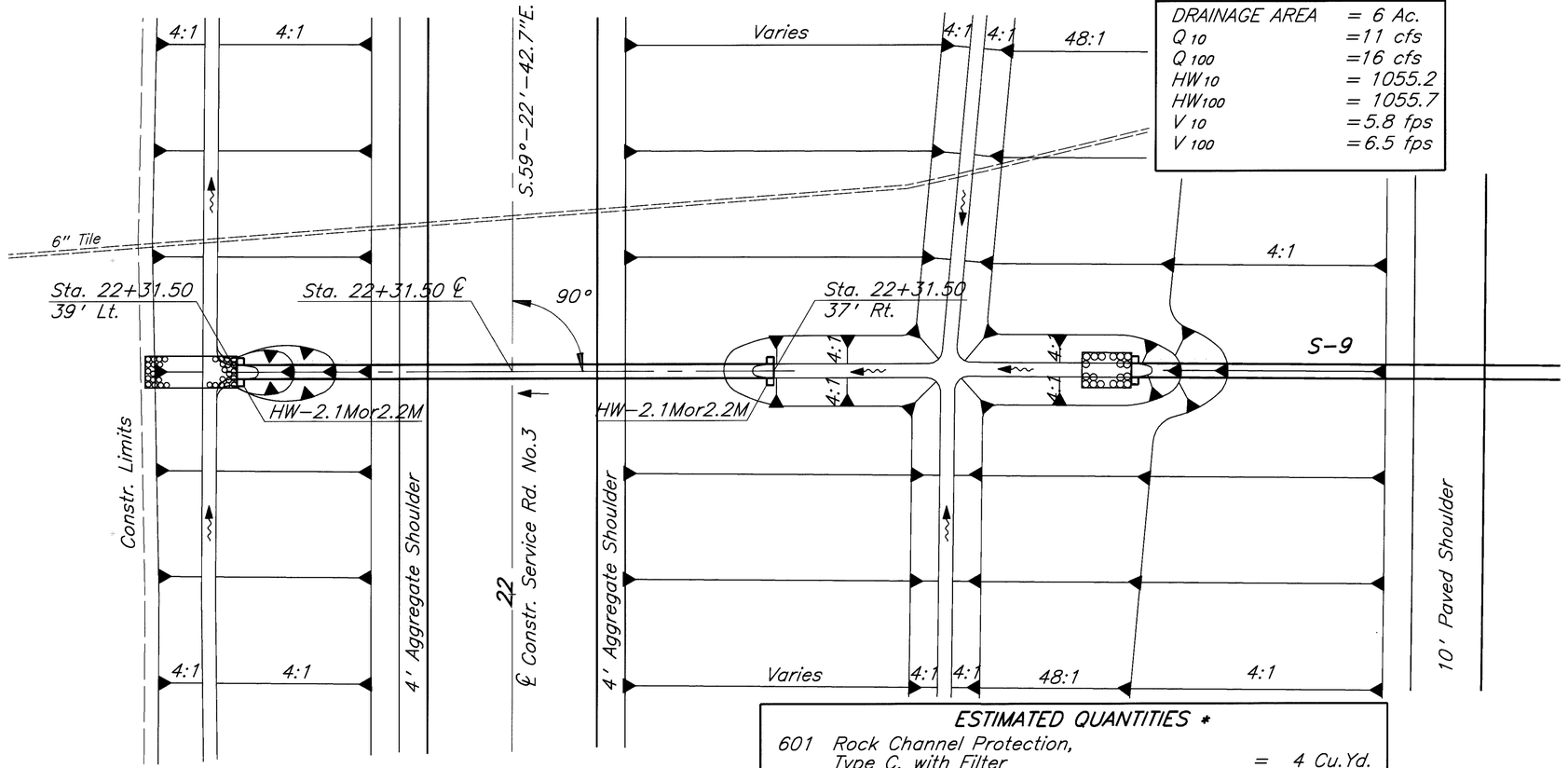
CALCULATED

CHECKED

HORIZONTAL SCALE IN FEET
0 5 10 20

CULVERT DETAIL S-21
SERVICE ROAD NO.3 STA. 19 + 34

FAY-35-2.57

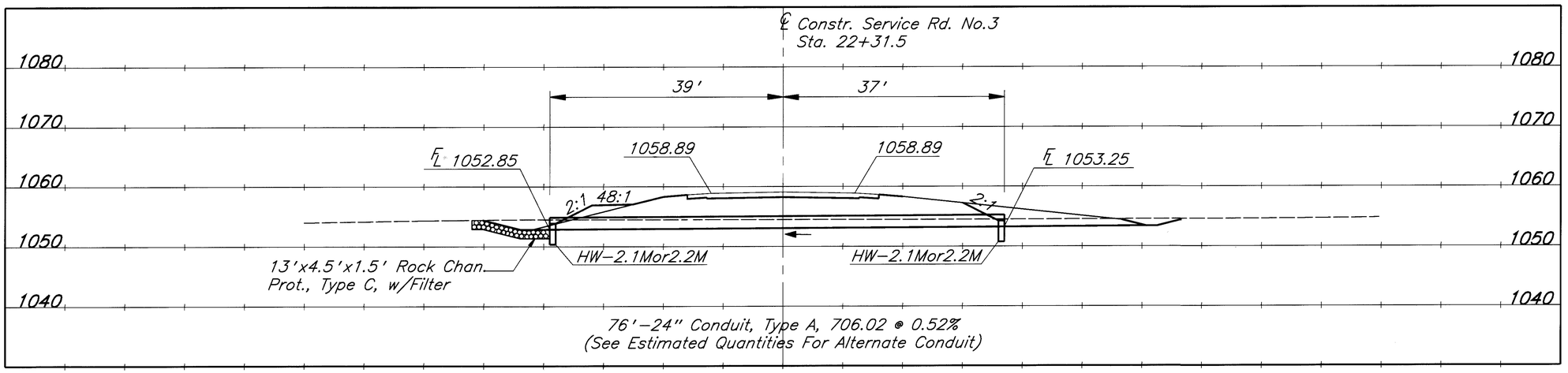


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	= 6 Ac.
Q 10	=11 cfs
Q 100	=16 cfs
HW10	= 1055.2
HW100	= 1055.7
V 10	=5.8 fps
V 100	=6.5 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 4 Cu.Yd.
602	Concrete Masonry	= 1.1 Cu.Yd.
603	24" Conduit, Type A, 706.02, or 706.08 or 27" Conduit, Type A, 707.05	= 76 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CULVERT DETAIL S-22
SERVICE ROAD NO.3 STA. 22 + 31.5

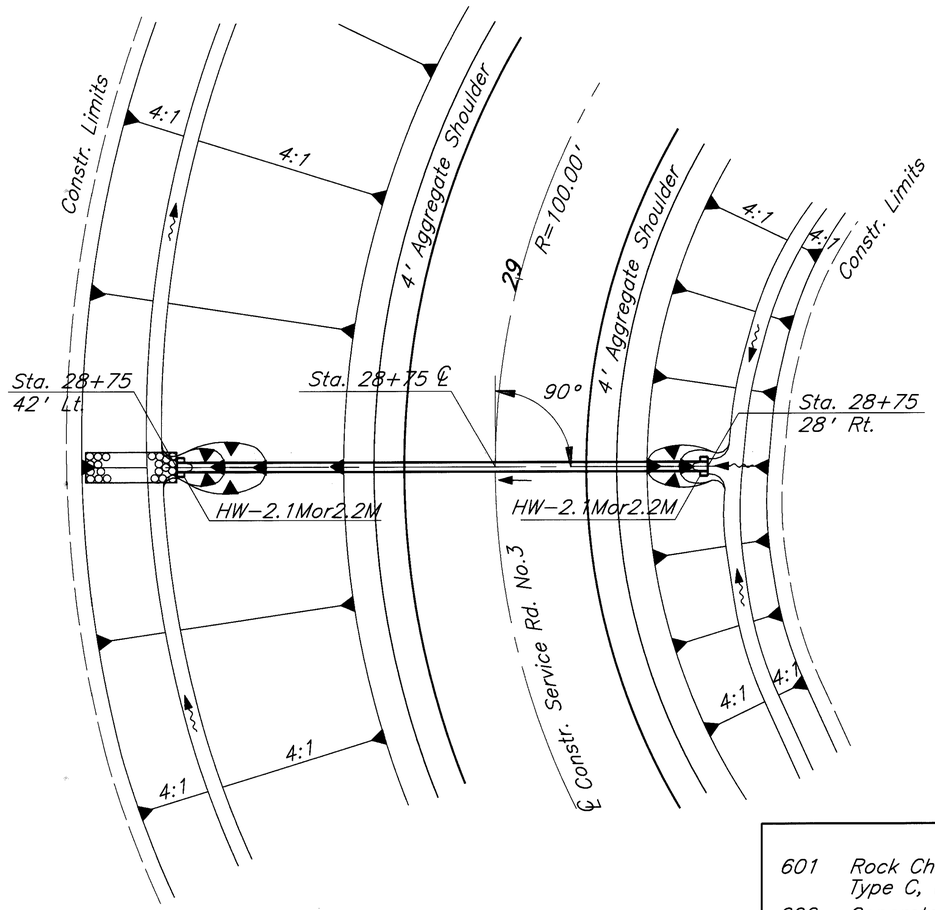
CALCULATED | CHECKED

0 5 10 20 HORIZONTAL SCALE IN FEET

FAY-35-2.57

S-22

310
522

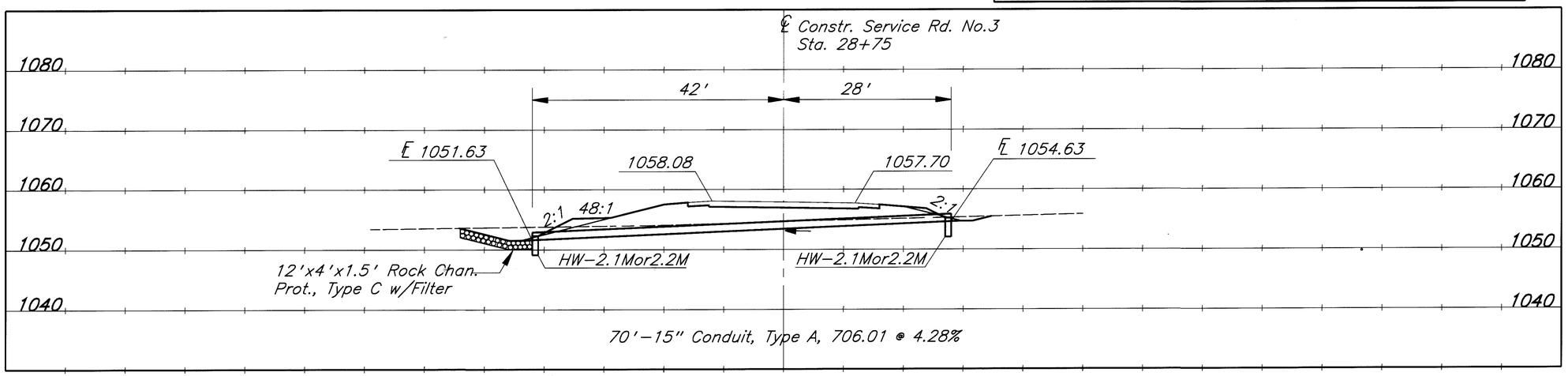


HYDRAULIC DESIGN DATA	
DRAINAGE AREA	=1.2 Ac.
Q 10	= 3 cfs
Q 100	= 5 cfs
HW 10	= 1055.7
HW 100	= 1056.2
V 10	=9.3 fps
V 100	=10.6 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 3 Cu.Yd.
602	Concrete Masonry	= 0.6 Cu.Yd.
603	15" Conduit, Type A, 706.01, 707.01 (.109),707.01 Aluminum Coated(.079), or 707.21	= 70 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CULVERT DETAIL S-23
SERVICE ROAD NO.3 STA. 28 + 75

CALCULATED

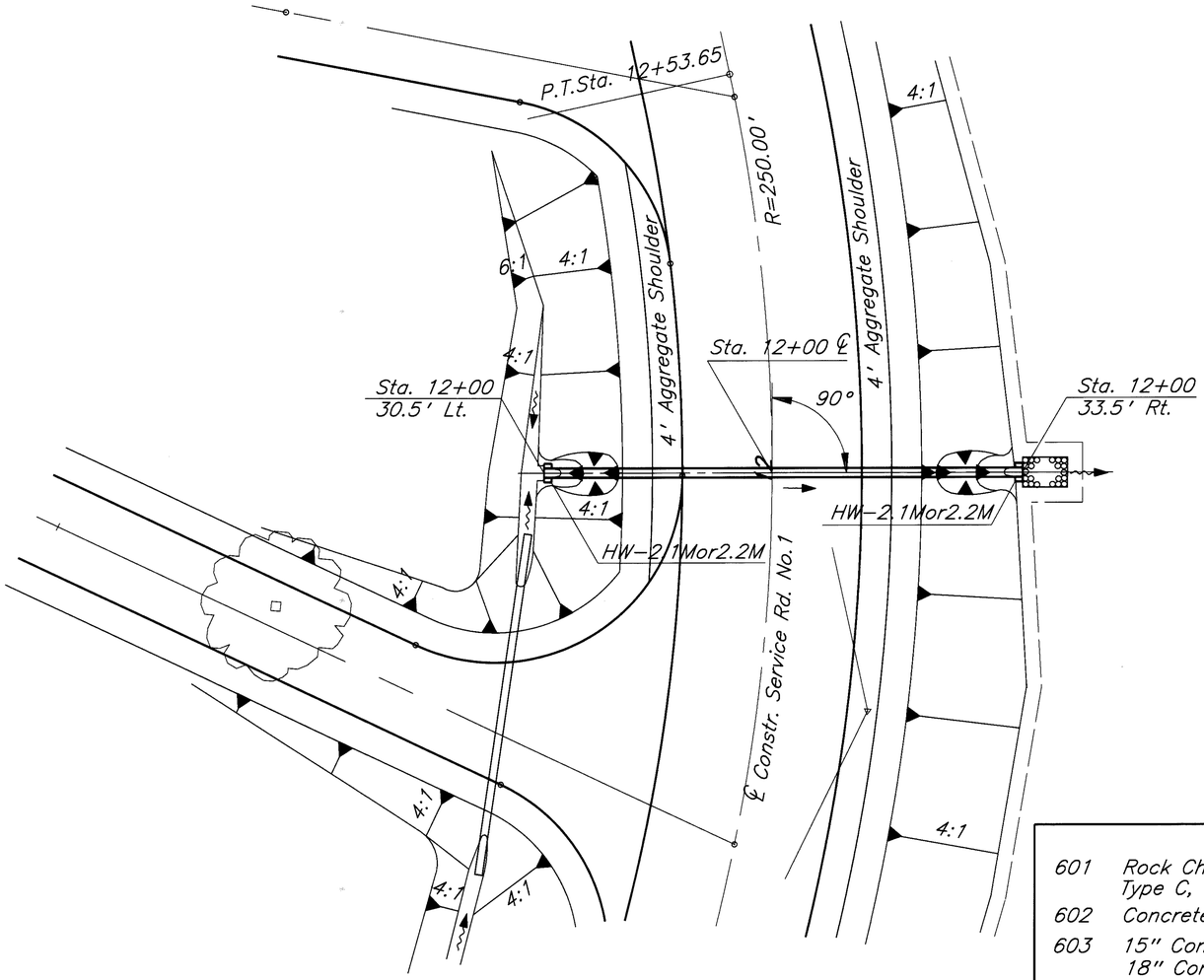
CHECKED

0 5 10 20 HORIZONTAL SCALE IN FEET

FAY-35-2.57

S-23

311
522



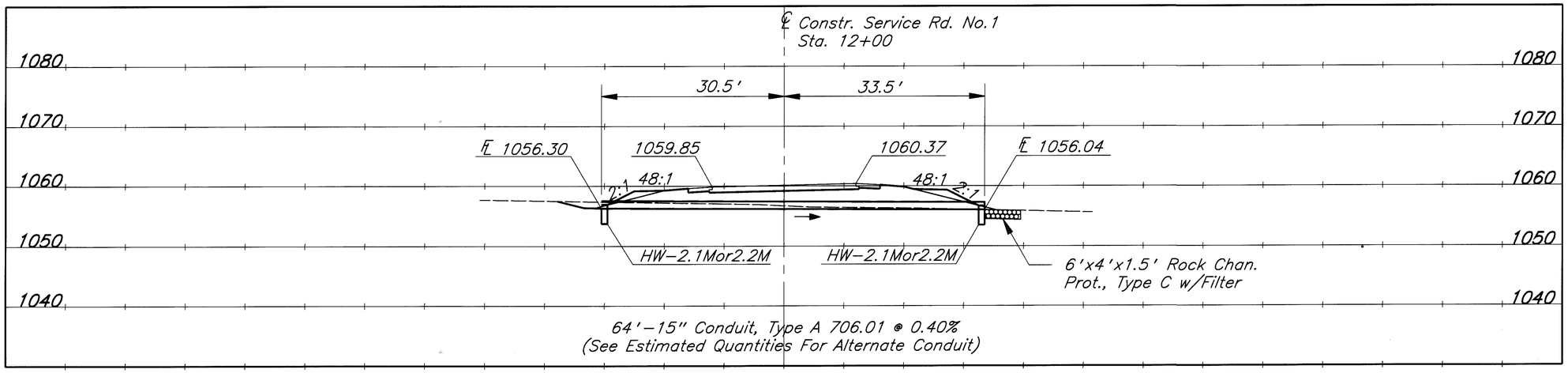
HYDRAULIC DESIGN DATA

DRAINAGE AREA	=1 Ac.
Q_{10}	= 3 cfs
Q_{100}	= 5 cfs
HW_{10}	= 1057.5
HW_{100}	= 1057.9
V_{10}	=4.3 fps
V_{100}	=5.2 fps

ESTIMATED QUANTITIES *

601	Rock Channel Protection, Type C, with Filter	= 2 Cu.Yd.
602	Concrete Masonry	= 0.7 Cu.Yd.
603	15" Conduit, Type A, 706.01 or 18" Conduit, Type A, 707.01(.109), 707.01 Aluminum Coated(.079), or 707.21	= 64 Lin.Ft.

* Carried to Culvert Subsummary Sheet



CALCULATED
CHECKED
0 5 10 20 HORIZONTAL SCALE IN FEET

CULVERT DETAIL S–24
SERVICE ROAD NO.1 STA. 12 + 00

FAY–35–2.57

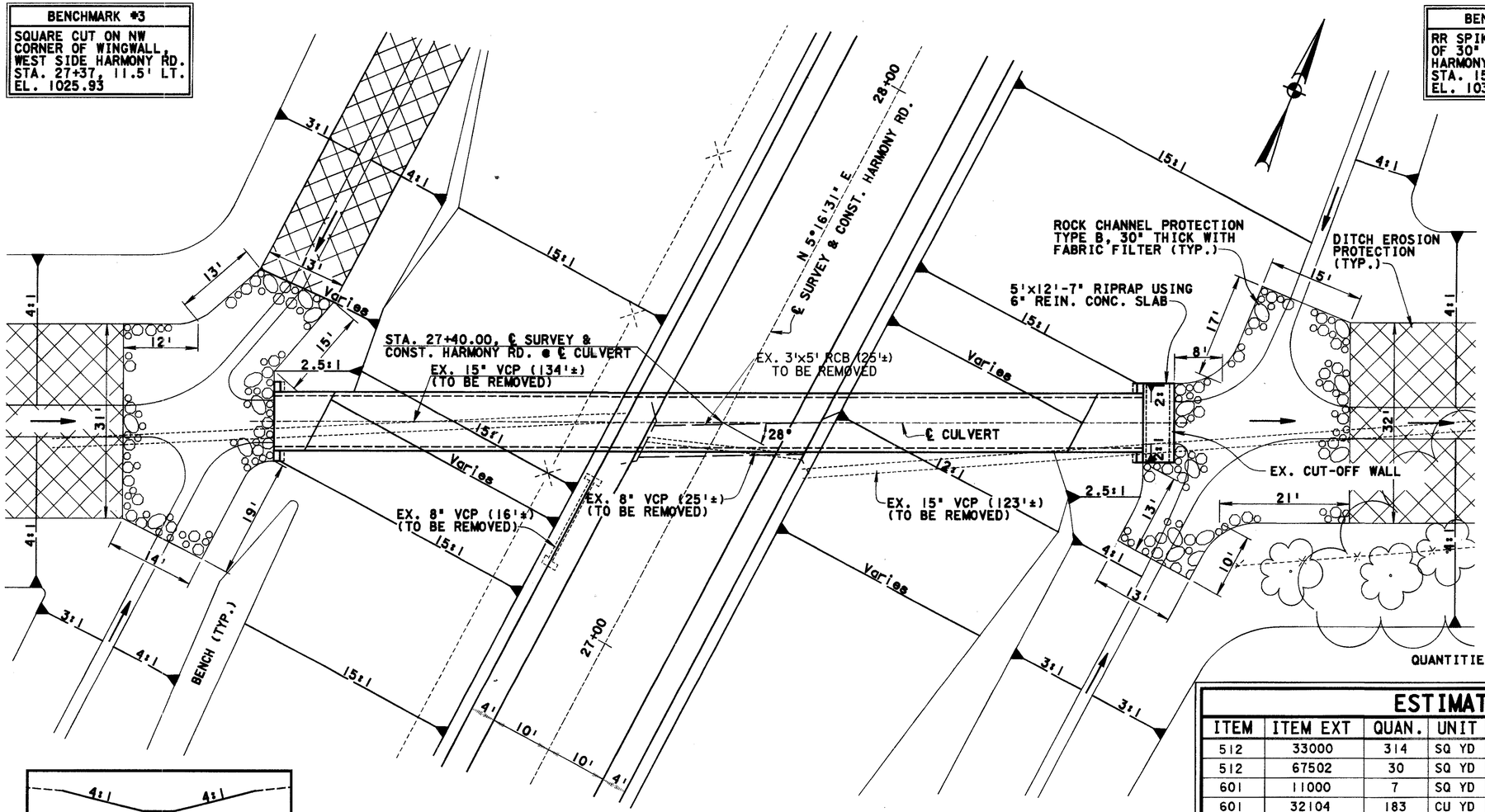
S–24

312
522

BENCHMARK #3
SQUARE CUT ON NW CORNER OF WINGWALL, WEST SIDE HARMONY RD.
STA. 27+37, 11.5' LT.
EL. 1025.93

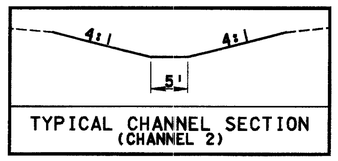
BENCHMARK #21
RR SPIKE WEST SIDE OF 30" OAK ALONG HARMONY ROAD
STA. 15+72, 24' RT.
EL. 1030.80

EARTHWORK: LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS-SECTIONS.



QUANTITIES CARRIED TO THE GENERAL SUMMARY SHEET 35.

PLAN



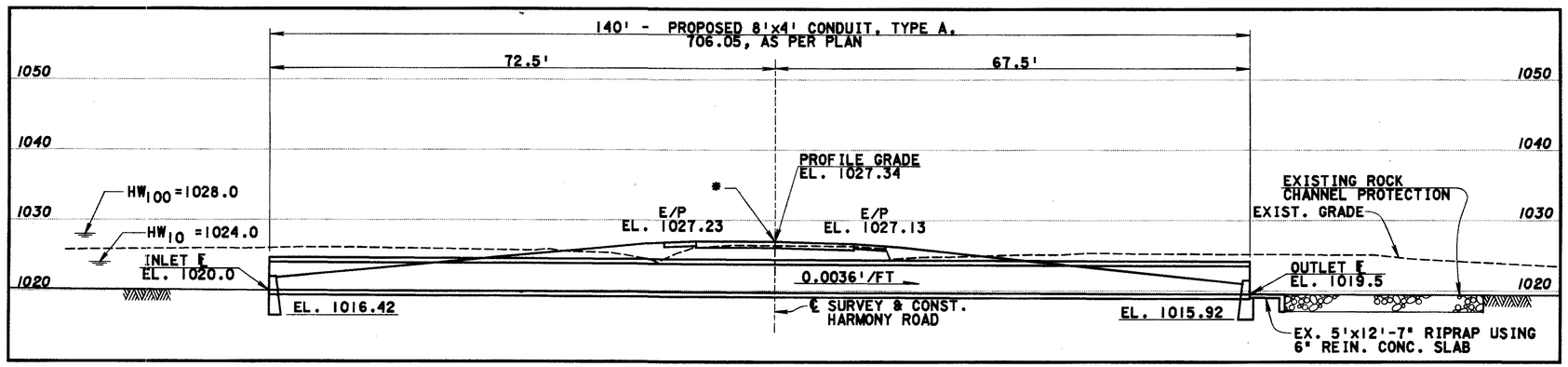
TYPICAL CHANNEL SECTION (CHANNEL 2)

NOTE:
FOR CHANNEL DETAILS, SEE CHANNEL 2 PLAN & PROFILE AND CROSS SECTION SHEETS 329 - 332A

ESTIMATED QUANTITIES

CALC. BY: EJC DATE: 8/28/95 CHK. BY: EBS DATE: 8/29/95

ITEM	ITEM EXT	QUAN.	UNIT	DESCRIPTION
512	33000	314	SQ YD	TYPE 2 WATERPROOFING
512	67502	30	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY)
601	11000	7	SQ YD	RIPRAP USING 6" REINFORCED CONCRETE SLAB
601	32104	183	CU YD	ROCK CHANNEL PROTECTION, TYPE B W/FABRIC FILTER
602	20001	5.7	CU YD	CONCRETE MASONRY, AS PER PLAN (Note on 312B)
603	94801	140	LIN FT	8'x4' CONDUIT, TYPE A, 706.05, AS PER PLAN



PROFILE
(ALONG ℄ CULVERT)

HYDRAULIC DESIGN DATA

DRAINAGE AREA: 0.7 SQ.MI.

DESIGN YEAR: Q_{10} = 175 CFS
HW_{10} = 1024.0
V_{10} = 10.42 FPS

HIGHWATER: ** Q_{100} = 320 CFS
HW_{100} = 1028.0
V_{100} = 11.11 FPS

PROPOSED STRUCTURE

TYPE: PRECAST REINFORCED CONCRETE BOX CULVERT
SIZE: 8' SPAN x 4' RISE
ROADWAY: 28'-0" F/F GUARDRAIL
SKEW: 28° R.F.
ALIGNMENT: TANGENT
CROWN: 0.016

* FOR TYPICAL SECTION OVER THE CULVERT SEE SHEET 11A / 522

** After road is revised with FAY-35-7.51 project.

S-25

DESIGN AGENCY: PARSONS BRINCKERHOFF OHIO INC. 655 METRO PLACE SOUTH, SUITE 390 DUBLIN, OHIO 43017
DESIGNED PJL | DRAWN EJC | REVIEWED EBS | DATE 7/19/95
CHECKED KDO | REVISED | STRUCTURE FILE NUMBER

FAYETTE COUNTY
HARMONY ROAD
STA. 27+40

CULVERT PLAN & PROFILE
PRECAST CONCRETE BOX CULVERT
UNDER HARMONY ROAD

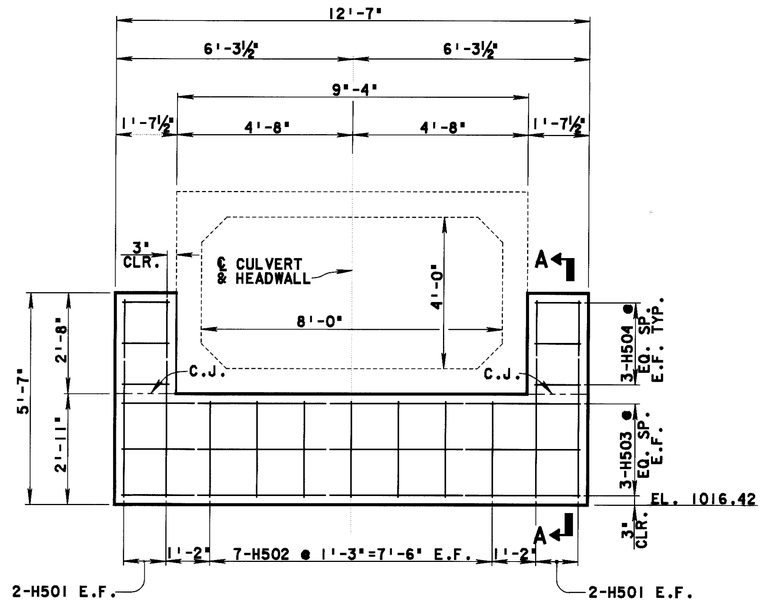
FAY-35-2.57

1 / 2

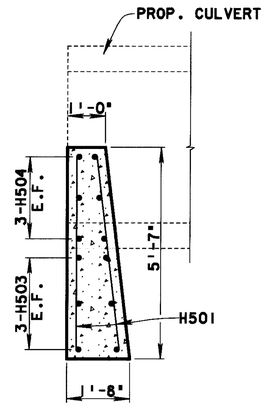
312A / 522

REVISED 11/99 BY FYC

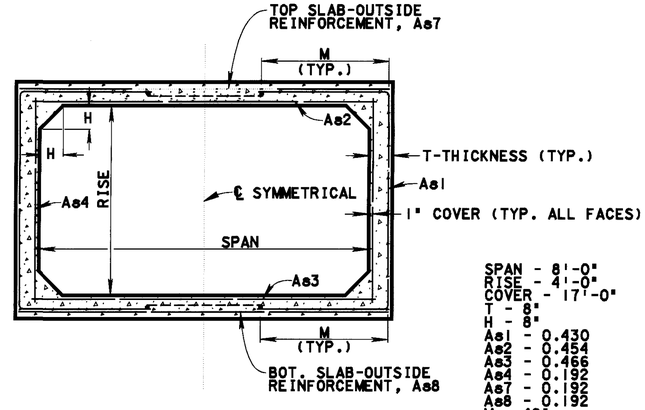
i:\pr\project\fay35\fay-35-2.57\ 06912sp1.dgn



INLET HEADWALL ELEVATION



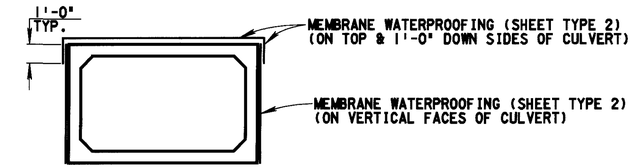
SECTION A-A



SPAN - 8'-0"
RISE - 4'-0"
COVER - 1'-0"
T - 8"
H - 8"
As1 - 0.430
As2 - 0.454
As3 - 0.466
As4 - 0.192
As7 - 0.192
As8 - 0.192
M - 42"

CULVERT SECTION
(SHOWING REINFORCING)

NOTES:
MAXIMUM SPACING OF REINFORCING SHALL BE 8".
THE MINIMUM CONCRETE COMPRESSIVE STRENGTH SHALL BE 5000 PSI. THE MINIMUM YIELD STRENGTH FOR REINFORCING SHALL BE 65000 PSI.
As MIN. = 0.002 x GROSS SECTION AREA



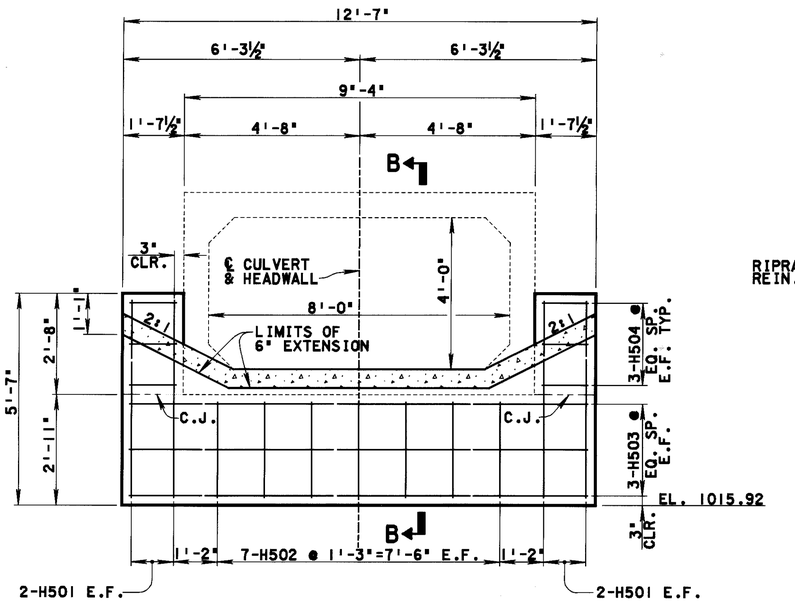
CULVERT SECTION
(SHOWING WATERPROOFING)

ITEM 603 CONDUIT TYPE, A, 706.05, AS PER PLAN

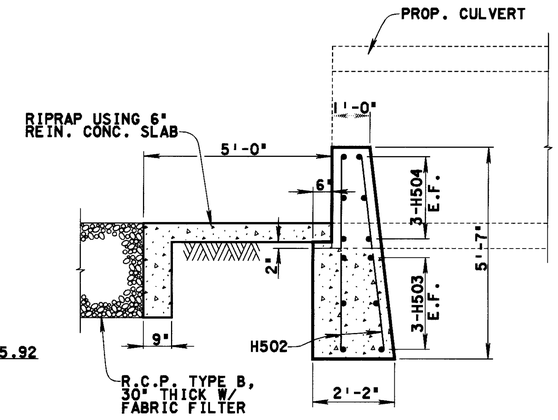
MEMBRANE WATERPROOFING (SHEET TYPE 2) SHALL BE APPLIED TO THE TOP SURFACE OF THE PRECAST CULVERT SECTIONS AND SHALL EXTEND VERTICALLY DOWN ALL SIDES FOR THE PORTIONS OF THE CULVERT WHICH SHALL BE IN CONTACT WITH THE BACKFILL. THE EXTERIOR JOINT GAP ON THE TOP AND SIDES BETWEEN THE PRECAST CULVERT SECTIONS SHALL BE FILLED WITH PORTLAND CEMENT MORTAR PRIOR TO INSTALLING THE MEMBRANE WATERPROOFING. JOINT WRAP AS SPECIFIED IN 603.06 AND CONCRETE SEALING AS SPECIFIED IN 603.08 ARE NOT REQUIRED UNDER THE LIMITS OF THE MEMBRANE WATERPROOFING. PAYMENT FOR THE MEMBRANE WATERPROOFING SHALL BE AT THE CONTRACT PRICE BID PER SQUARE YARD FOR ITEM SPECIAL - MEMBRANE WATERPROOFING (SHEET TYPE 2).

WHEN SEALING OF CONCRETE SURFACES (EPOXY) IS SPECIFIED ON THE HEADWALLS OF A PRECAST CONCRETE BOX CULVERT, ANY PRECAST CULVERT SECTIONS BEYOND THE LIMIT OF THE MEMBRANE WATERPROOFING SHALL BE SEALED USING EPOXY SEALER. PAYMENT FOR THE SEALING OF THE PRECAST CONCRETE BOX SURFACES SHALL BE MADE AT THE CONTRACT PRICE BID PER SQUARE YARD FOR ITEM SPECIAL - SEALING OF CONRETE SURFACES (EPOXY).

NOTE: THE FURNISHING AND INSTALLATION OF THE REINFORCING STEEL IS TO BE INCLUDED IN THE PRICE BID FOR ITEM 602, CONCRETE MASONRY, AS PER PLAN.



OUTLET HEADWALL ELEVATION



SECTION B-B

REINFORCING STEEL LIST●

MARK	NUMBER OF BARS TOTAL	INLET	OUTLET	LENGTH	WEIGHT	TYPE	A	B	C
HEADWALLS									
H501	16	8	8	5'-2"	87	STR			
H502	28	14	14	2'-6"	73	STR			
H503	12	6	6	12'-3"	154	STR			
H504	24	12	12	1'-3"	32	STR			

● NOTE: ALL BARS TO BE EPOXY COATED.

TOTAL FOR ALL REBARS= 346 LBS.

ABBREVIATIONS
C.J. - CONSTRUCTION JOINT
E.F. - EACH FACE
EQ. SP. - EQUAL SPACES
CLR. - CLEAR
TYP. - TYPICAL

DESIGN AGENCY
PARSONS BRINCKERHOFF OHIO INC.
655 METRO PLACE SOUTH, SUITE 390
DUBLIN, OHIO 43017

DATE 8/29/95
REVIEWED EBS
STRUCTURE FILE NUMBER
DRAWN EJC
REVISED
DESIGNED EJC
CHECKED PJL

CULVERT DETAILS
PRECAST CONCRETE BOX CULVERT UNDER HARMONY ROAD

FAY-35-2.57

2/2

312B
522

i:\pr\project\fay35\fay-35-2.57\ 06912sp2.dgn

STORM SEWER SUB-SUMMARY

REFERENCE NUMBER	STORM SHEET PROFILE SHEET NO.	STRUCTURES INCLUDED	SIDE		601		602		603	603			603	603	603			603	603	603	604	604	604		604	604		604
					ROCK CHANNEL PROTECTION, TYPE C W/FILTER		CONCRETE MASONRY		15" CONDUIT, TYPE B	15" CONDUIT, TYPE F			18" CONDUIT, TYPE B	21" CONDUIT, TYPE C	24" CONDUIT, TYPE B			30" CONDUIT, TYPE B	30" CONDUIT, TYPE C	36" CONDUIT, TYPE C	CATCH BASIN NO. 3A	CATCH BASIN NO. 4	CATCH BASIN NO. 4A		CATCH BASIN NO. 8	CATCH BASIN NO. 8A		CATCH BASIN NO. 2-2B
					CU.YD.		CU.YD.		LIN.FT.	LIN.FT.			LIN.FT.	LIN.FT.	LIN.FT.			LIN.FT.	LIN.FT.	LIN.FT.	EACH	EACH	EACH		EACH	EACH		EACH
P-1	315	H-1,C-1	LT/RT		2		0.3		107																			1
P-2	315	C-2	RT		2		0.3		106													1						
P-4	315	C-4	RT						86													1						
P-5	315	C-5	RT						78													1						
P-6	315	C-6	RT						78													1						
P-8	316	H-7,C-8	RT		3		0.8													120						1		
P-9	316	C-9	RT																	300						1		
P-10	316	C-10	RT																	300						1		
P-11	316	C-11	RT																300							1		
P-12	316	C-12	RT																300							1		
P-13	316	C-13	RT																300							1		
P-14	316	C-14	RT																300							1		
P-15	316	C-15	RT																300							1		
P-16	316	C-16	RT											275											1			
P-17	316	C-17	RT											300											1			
P-18	316	C-18	RT											300											1			
P-19	316	C-19	RT						86													1						
P-20	317	H-20,C-20	RT		3		0.3		127													1						
P-21	317	H-21,C-21	LT		2		0.3		74	35												1						
P-22	317	H-22,C-22	LT		2		0.3		108													1						
P-23	317	H-23,C-23	LT		2		0.3		99													1						
P-24	317	H-24,C-24	LT/RT		2		0.3		48	59																		1
P-25	318	H-25,C-25	LT		2		0.4						90									1						
P-26	318	H-26,C-26	RT		3		0.3		115													1						
P-27	318	H-27,C-27	RT		2		0.3		109													1						
P-28	318	H-28,C-28	LT		3		0.3		98													1						
P-30	319	H-30,C-30	LT		6		0.6											99					1					
P-31	319	C-31	RT															84								1		
P-32	319	H-32,C-32	LT		4		0.3		119													1						
P-33	319	H-33,C-33	LT		3		0.3		107													1						
P-34	320	H-34,C-34	LT		3		0.3		103													1						
P-35	320	H-35,C-35	LT		3		0.3		112													1						
P-36	320	H-36,C-36	LT		5		0.3		79	54												1						
P-37	320	H-37,C-37	LT		5		0.3		100													1						
P-38	320	H-38,C-38	LT		5		0.3		105													1						
P-39	321	H-39,C-39	LT		5		0.3		108													1						
P-40	321	H-40,C-40	LT		5		0.3		110													1						
P-41	321	H-41	LT				0.5								90													
P-42	321	H-42,C-42	RT		2		0.5								96							1						
P-46	322	H-46,C-46	RT		2		0.3			68											1							
P-48	322	H-48,C-48	RT		4		0.3			62											1							
P-59	322	H-59,C-59	LT		2		0.3			68											1							
P-60	322	H-60,C-60	RT		2		0.3			61											1							
SUB-TOTALS					84		9.7		2262	407			90	875	186			183	1500	720	4	23	1		3	9		2

CALCULATED

CHECKED

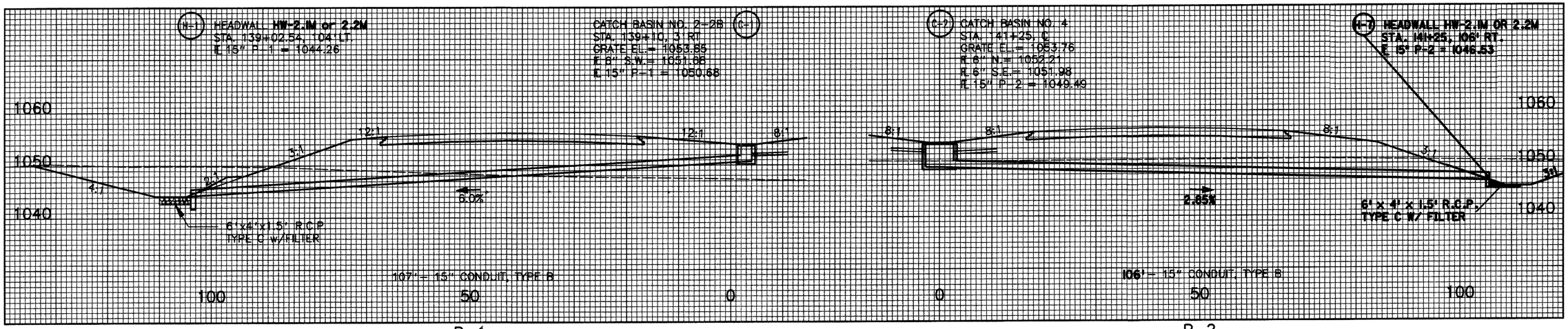
STORM SEWER SUB-SUMMARY

FAY-35-2.57

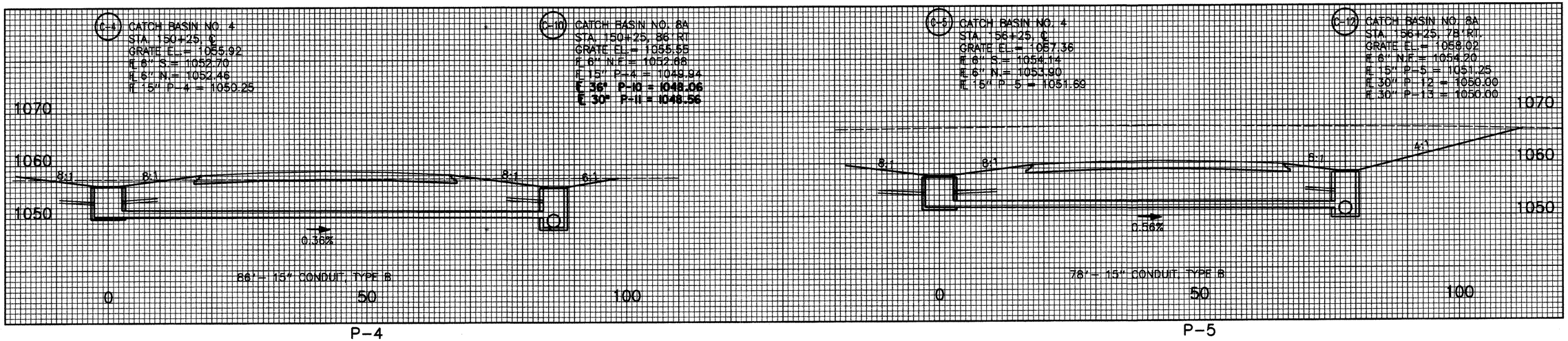
313
522

REVISED 1-31-01 DSW

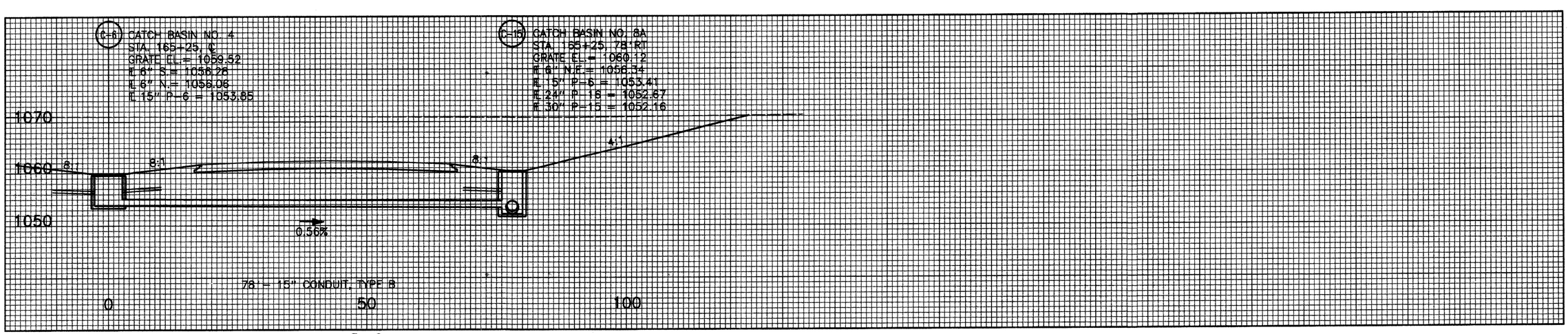
i:\users\dwise\fay\35\257&75i\40inewsht\new313.dgn 20-MAR-2001 3:40PM dwise



P-1 P-2



P-4 P-5



P-6

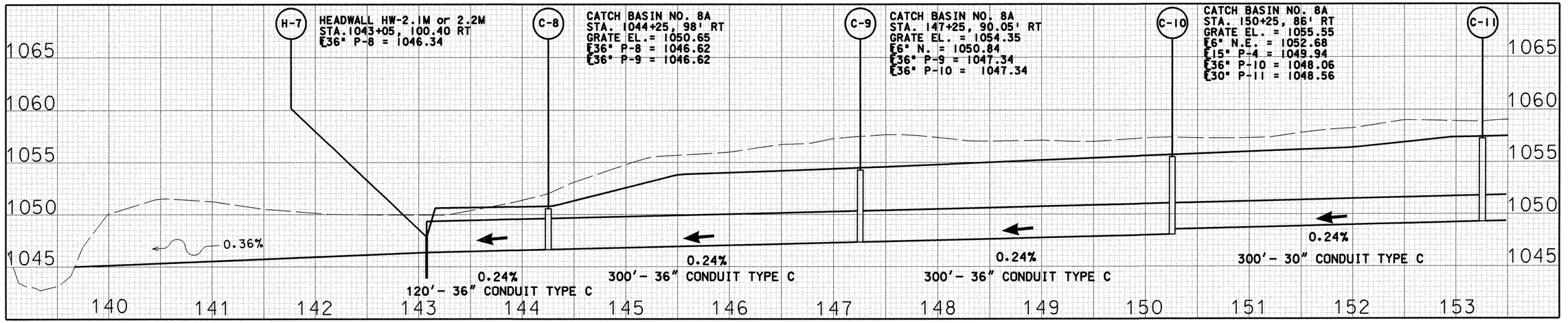
CALCULATED
CHECKED

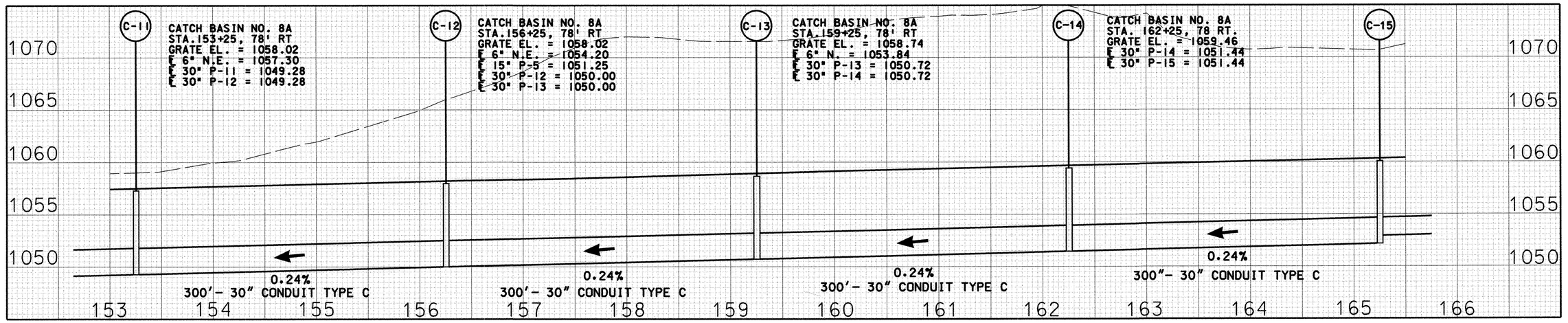
STORM SEWER PROFILES P-1 TO P-6

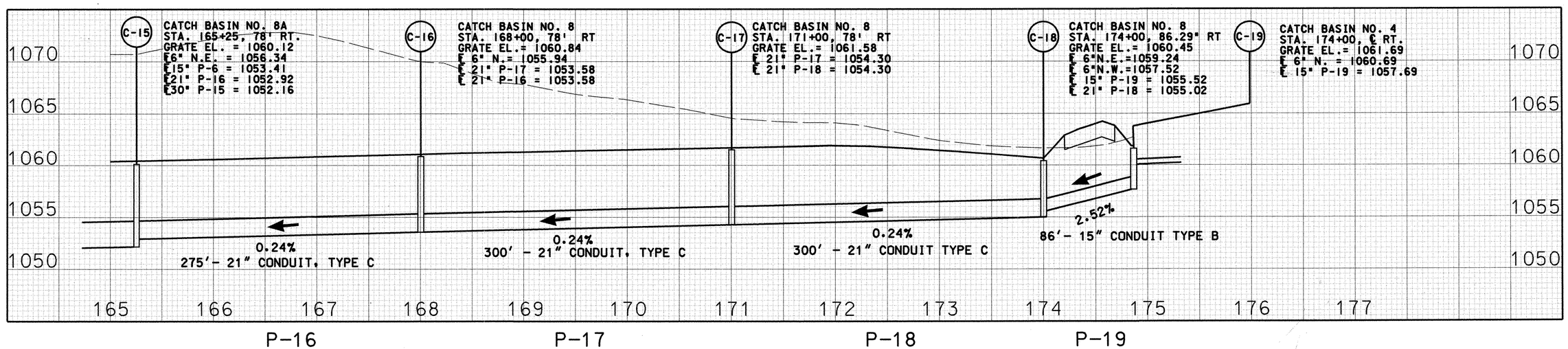
FAY-35-2.57

REVISED 1-31-01 DSW

i:\users\dwise\fay\35\257&751\40lnewsht\new315.dgn 20-MAR-2001 3:41PM dwise







CALCULATED DSW | CHECKED DSW

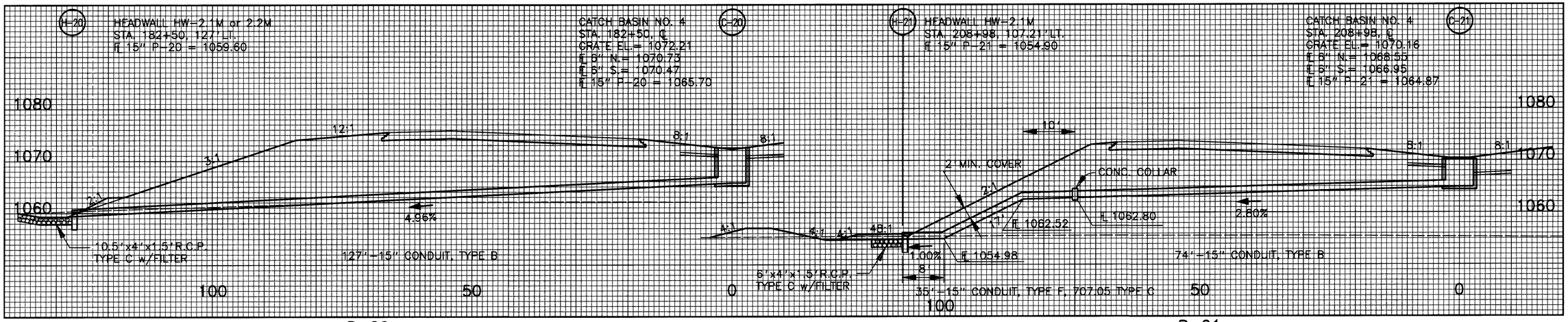
REVISED 01-31-01 DSW

STORM SEWER PROFILES
P-8 TO P-19

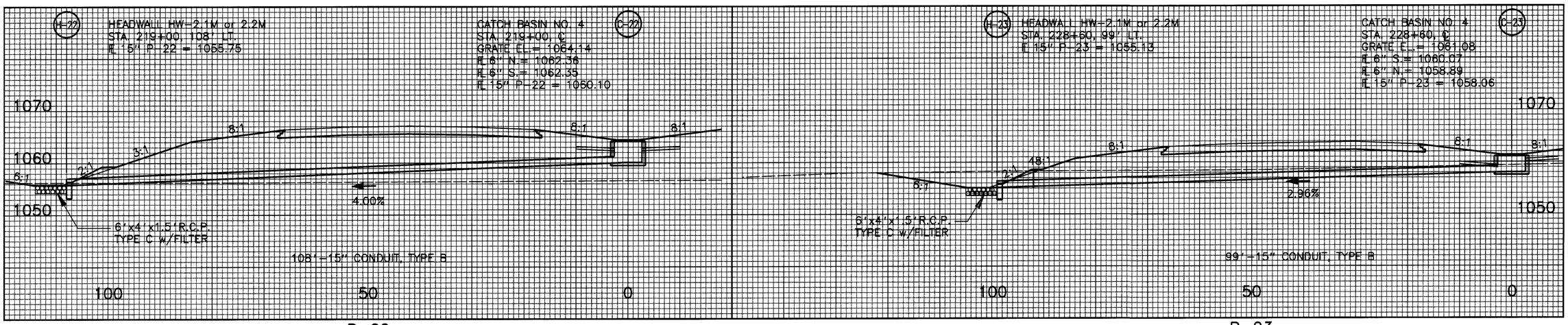
FAY-35-2.57

316
522

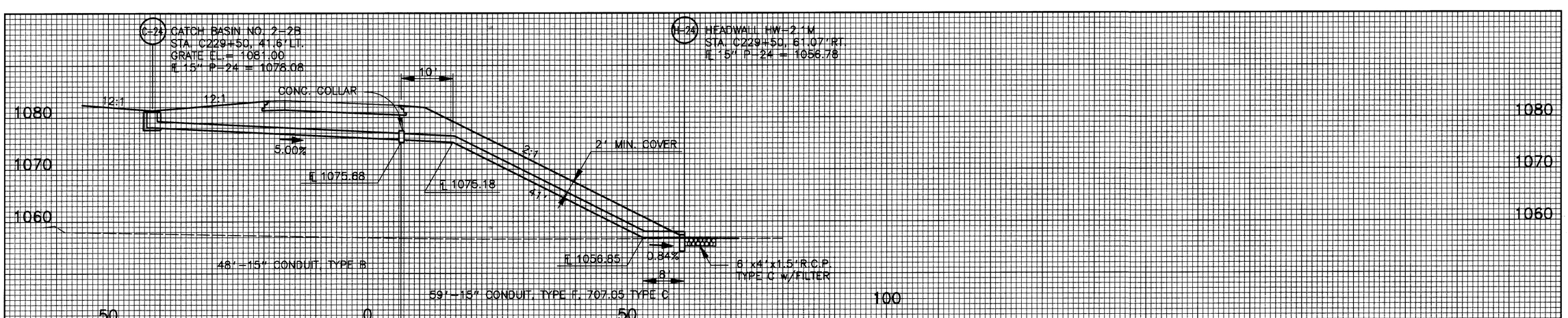
i:\users\dwise\fay\35\257&75\40lnewsht\new316.dgn 20-MAR-2001 3:41PM dwise



P-20 P-21



P-22 P-23

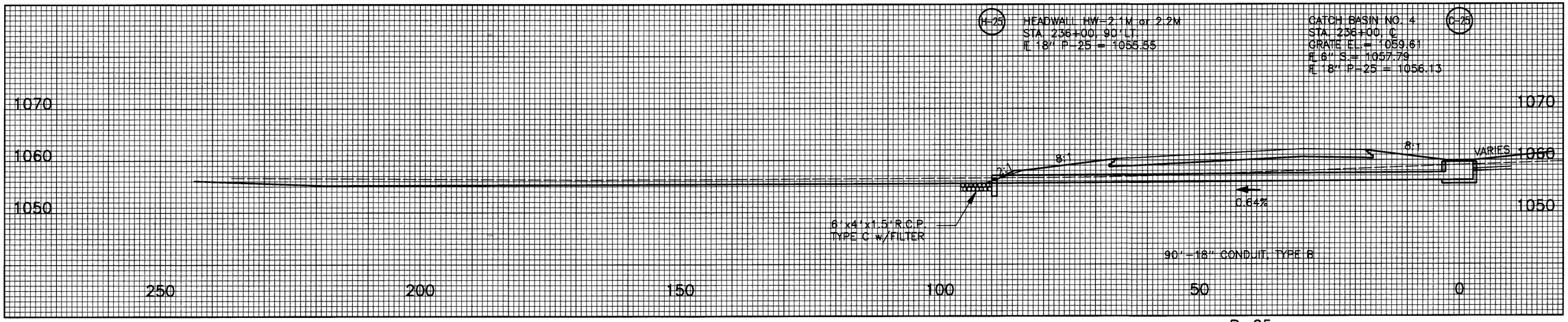


P-24

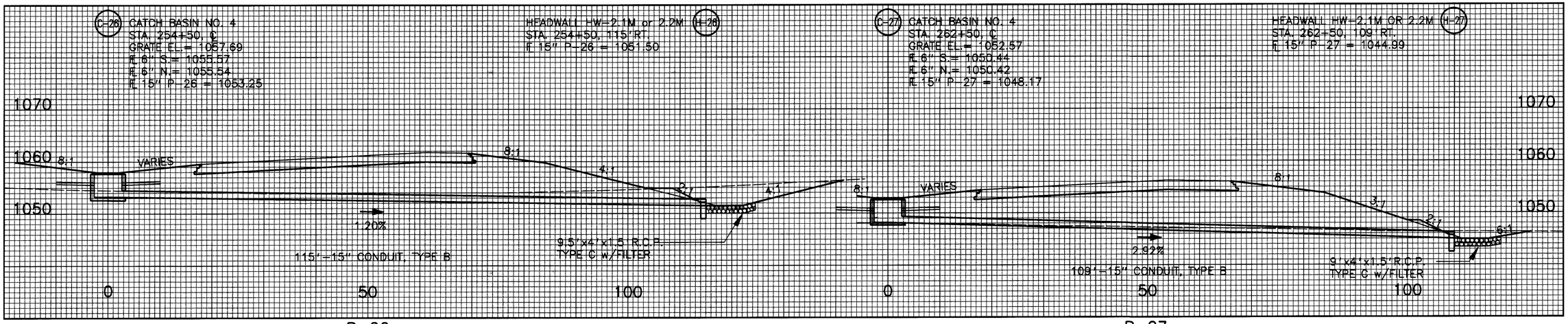
CALCULATED
CHECKED

STORM SEWER PROFILES P-20 TO P-24

FAY-35-2.57

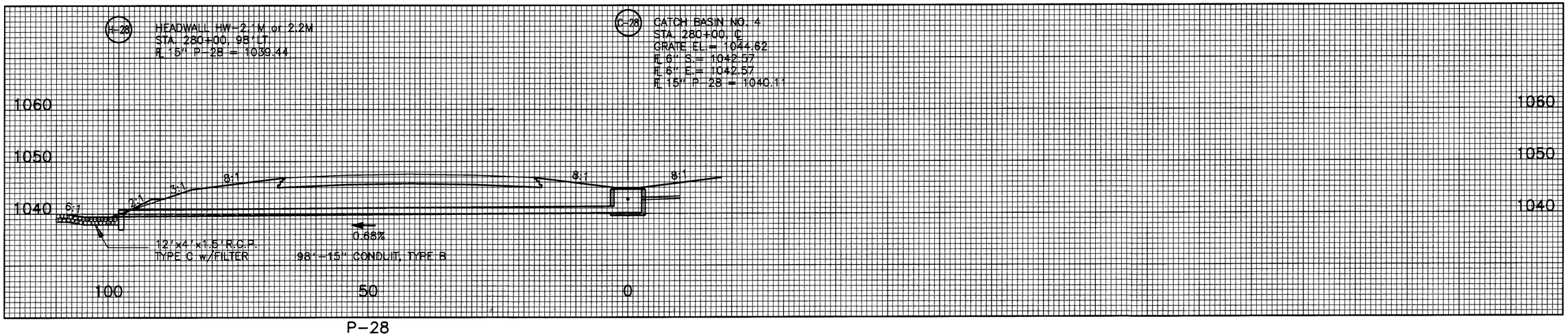


P-25



P-26

P-27



P-28

STORM SEWER PROFILES P-25 TO P-28

FAY-35-2.57

CALCULATED

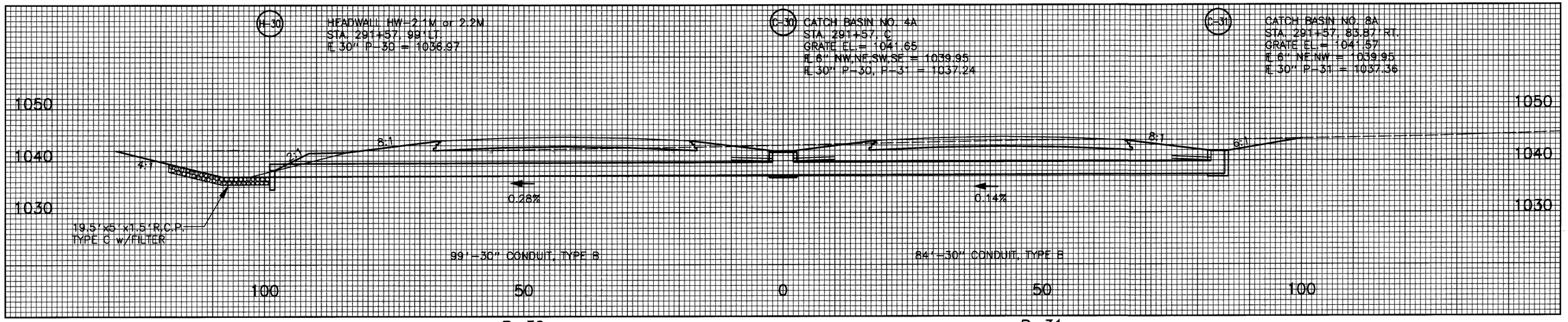
CHECKED

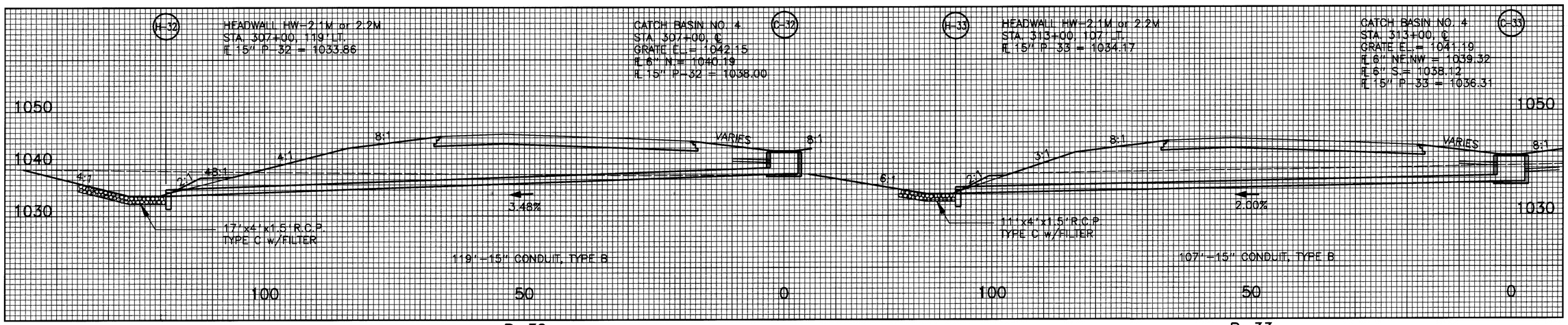
318
522

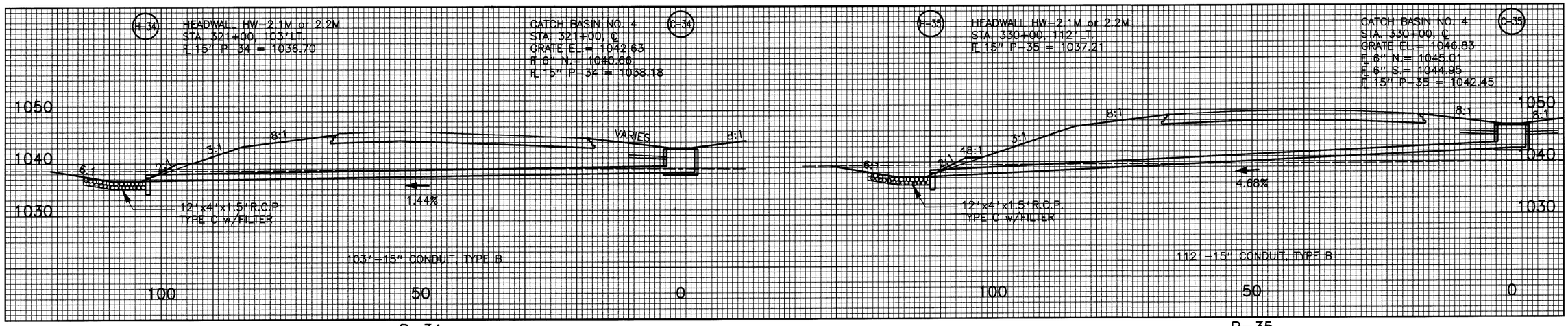
CALCULATED | CHECKED

STORM SEWER PROFILES P-30 TO P-33

FAY-35-2.57

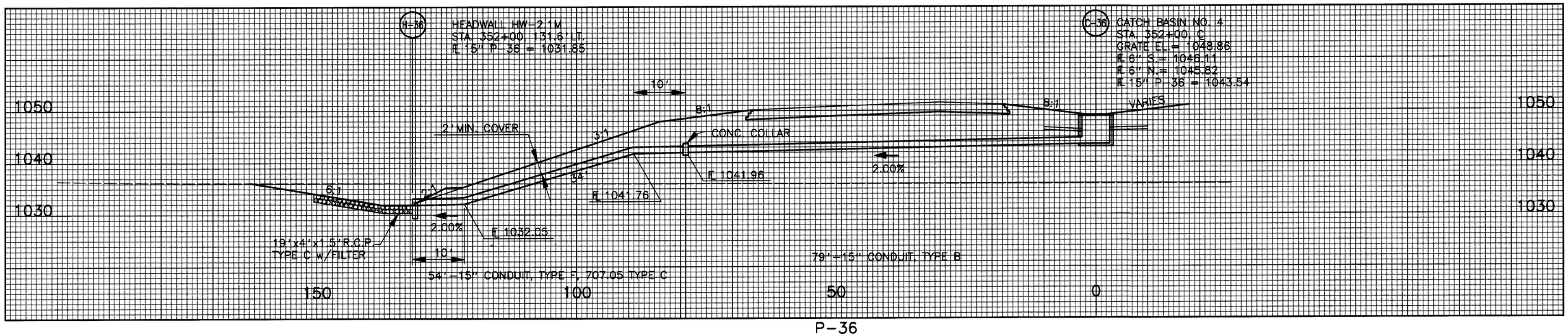




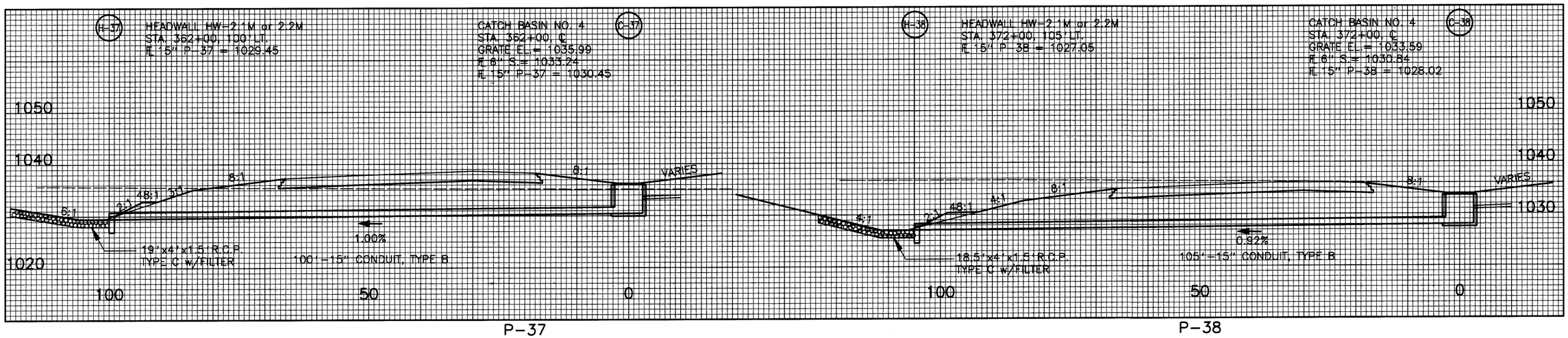


P-34

P-35



P-36



P-37

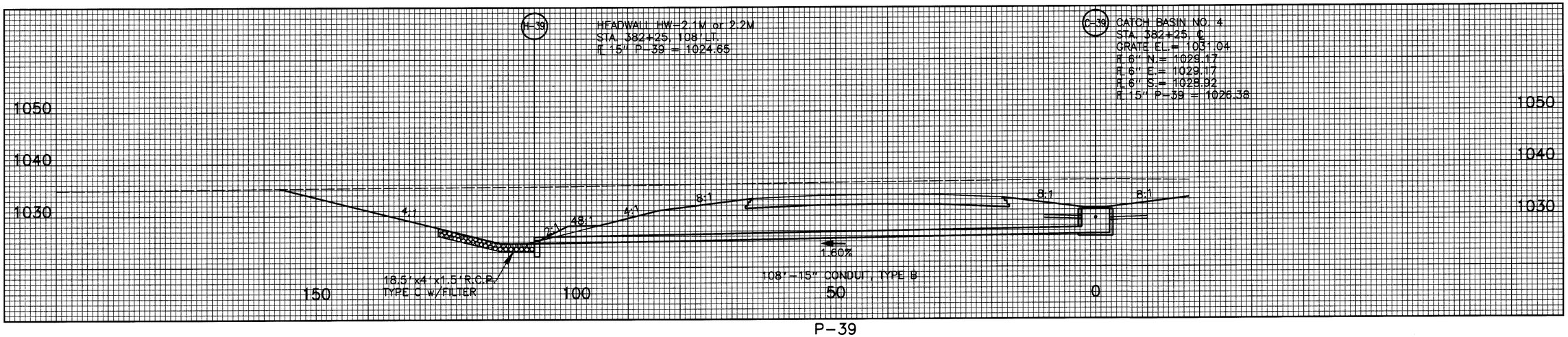
P-38

CALCULATED

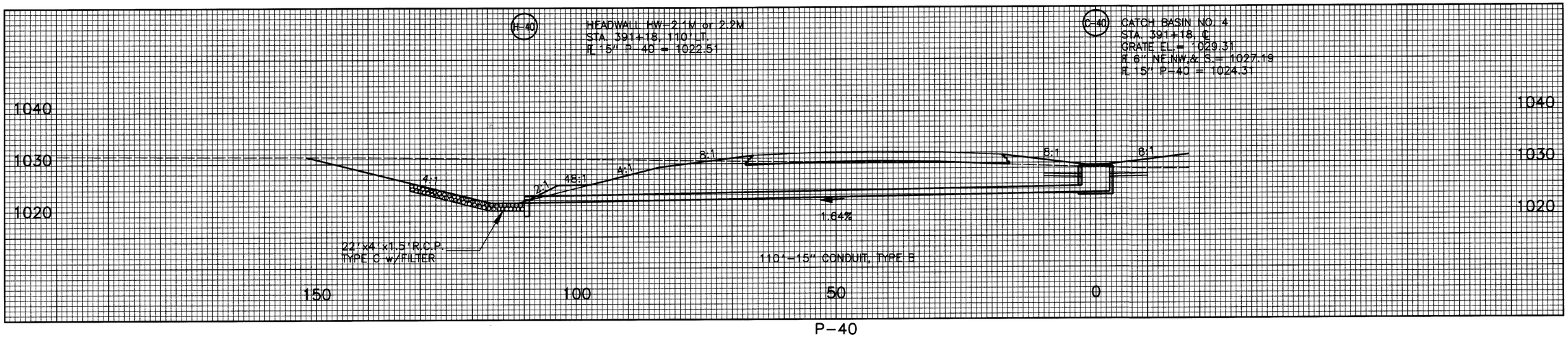
CHECKED

STORM SEWER PROFILES P-34 TO P-38

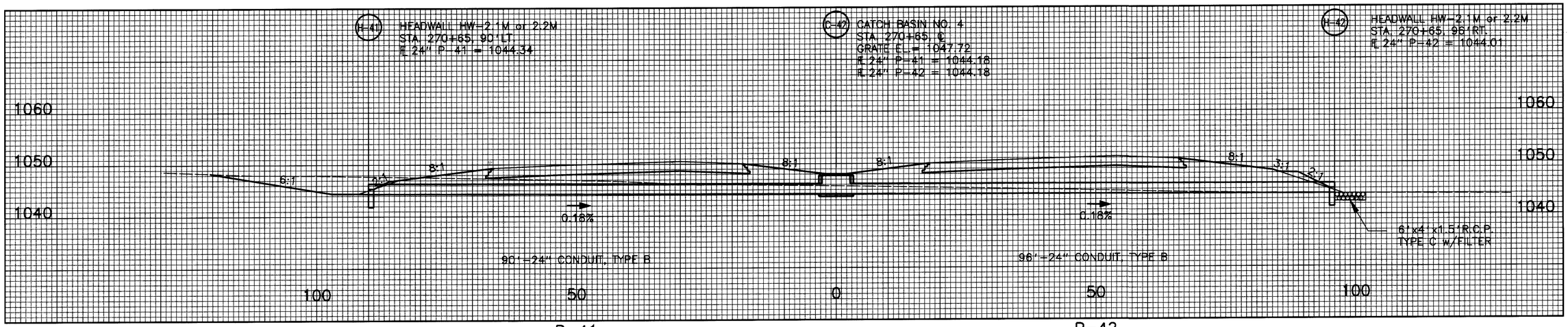
FAY-35-2.57



P-39



P-40



P-41

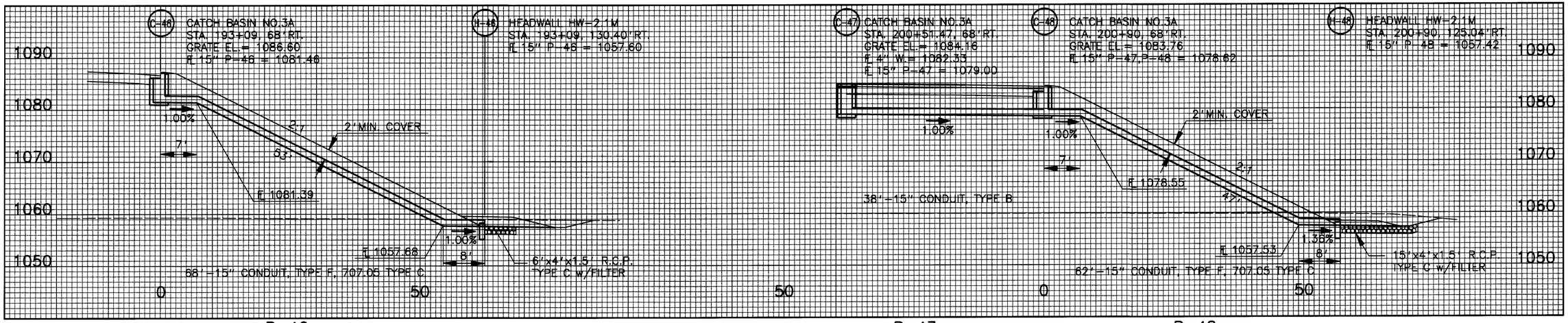
P-42

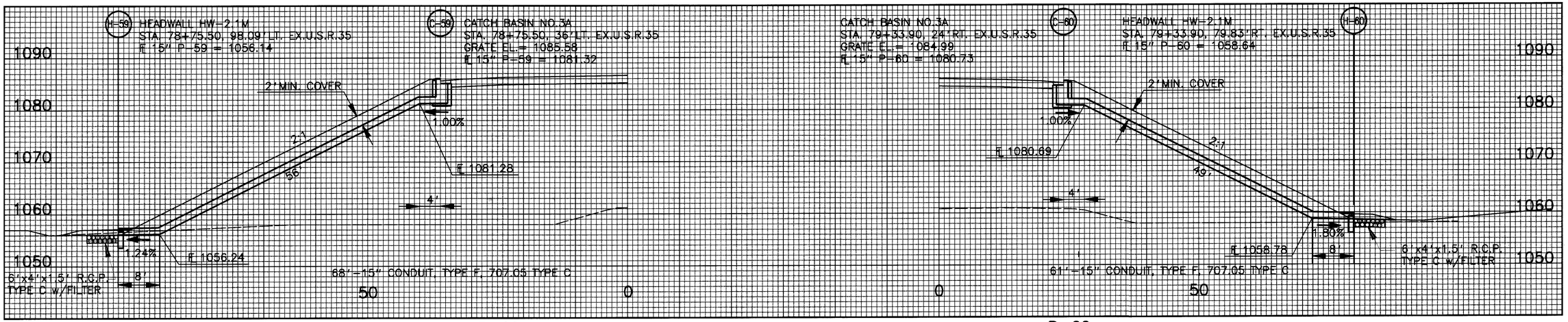
CALCULATED | CHECKED

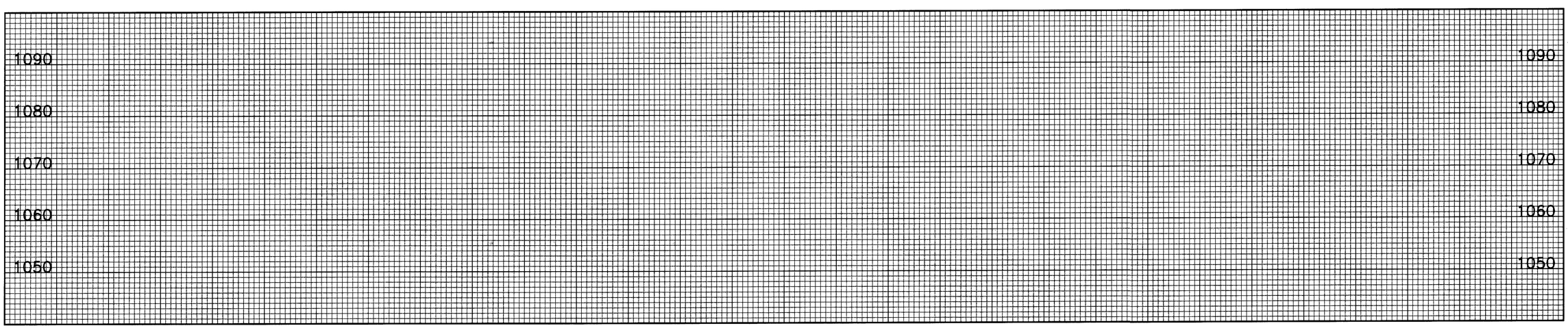
STORM SEWER PROFILES P-39 TO P-42

FAY-35-2.57

321
522





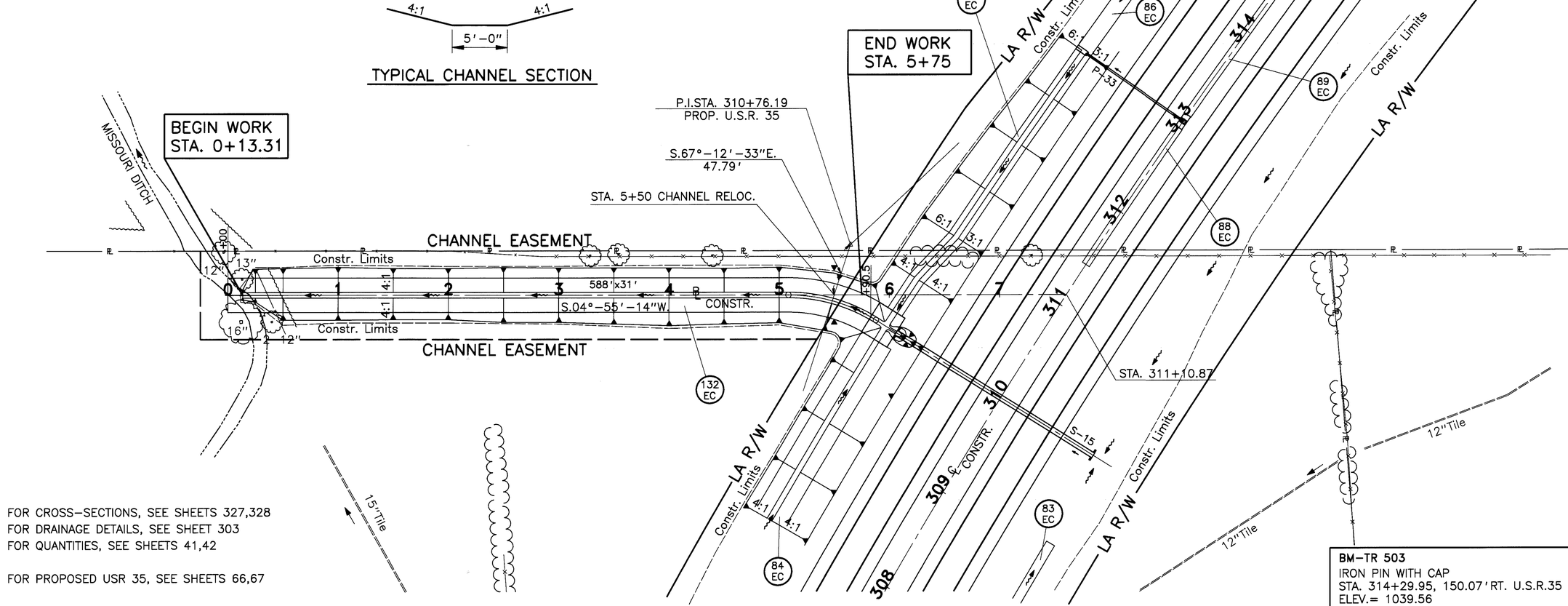


CALCULATED | CHECKED

STORM SEWER PROFILES P-46, P-48, P-59, P-60

FAY-35-2.57

322
522



CALCULATED

CHECKED

PLAN & PROFILE CHANNEL 1

FAY-35-2.57

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.C.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
50	556	131	0	499	0	
50	286	138	0	246	0	
53	146	127	0	93	0	
52	67	73	0	17	0	
51		2	0			

PROP. CHANNEL EASE.

4:1

1032.4
STATION 2+00.00
1037.42

1032.2
STATION 1+00.00
1037.35

1032.1
STATION 0+50.00
1036.80

1032.15
STATION 0+25.00
1035.62

1032.0
STATION 0+13.31
1032.00

BEGIN WORK 0+13.31

3590 CHANNEL 1 X-SECTIONS STA. 0+13.31 TO STA. 2+00

FAY-35-2.57

327
522

Q:\3590\3590C1X1

SEEDING		END AREA		VOLUME		CALCULATED
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	J.L.G.
						CHECKED
						W.G.Y.
24	77	184	0	171	0	
31	225	184	0	315	0	
50	556	156	0	608	0	
50	556	172	0	582	0	
50	556	142	0	506	0	
50		131	0			

130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120

1050 / 1040 / 1030

PROP. LA R/W

STA 5+75 – MEET MAINLINE CHANNEL STA310+00

4:1 4:1

1033.15
STATION 5+75
1039.27

PROP. CHANNEL EASE.

PROP. LA R/W

1033.1
STATION 5+50.00
1039.16

PROP. CHANNEL EASE.

PROP. CHANNEL EASE.

1033.0
STATION 5+00.00
1038.62

PROP. CHANNEL EASE.

PROP. CHANNEL EASE.

1032.8
STATION 4+00.00
1038.79

PROP. CHANNEL EASE.

PROP. CHANNEL EASE.

4:1 4:1

1032.6
STATION 3+00.00
1038.00

STA. 2+00

STA. 2+00

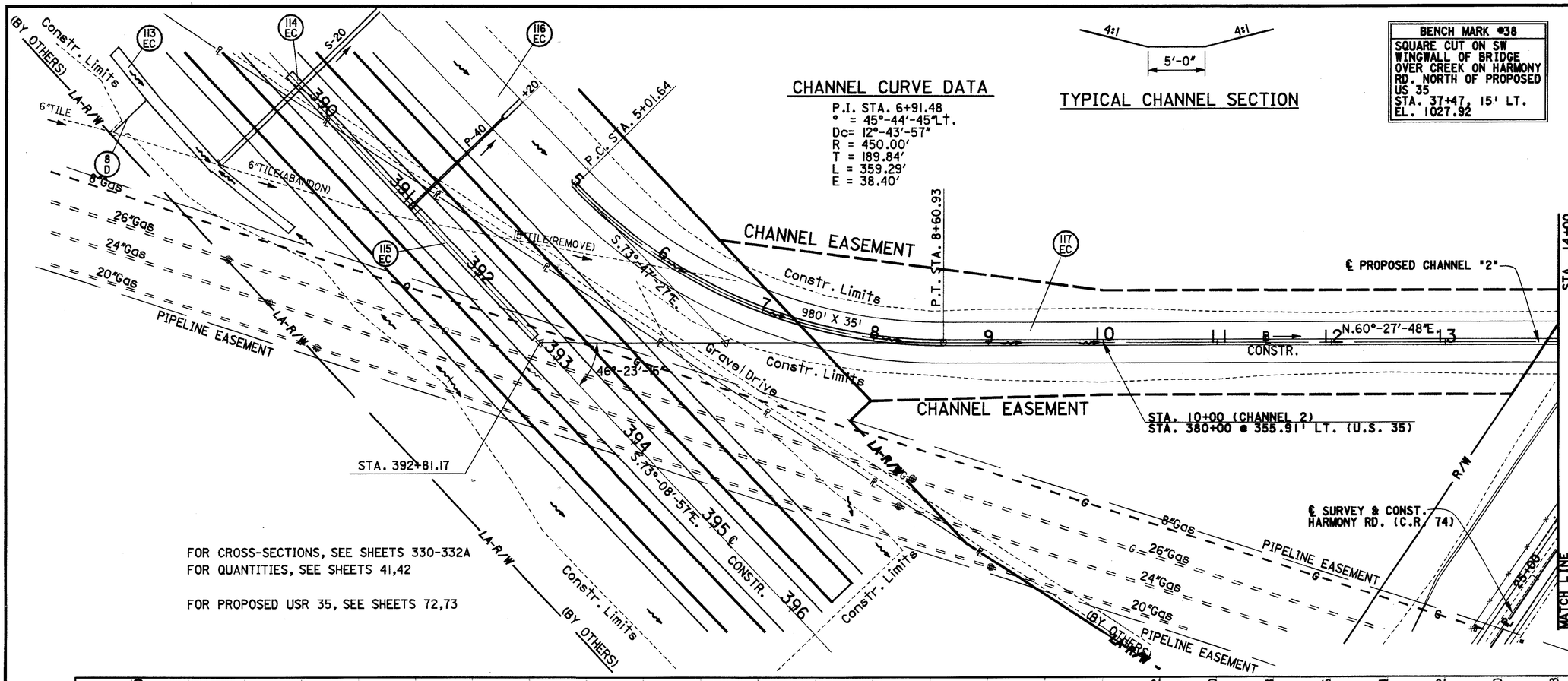
130 120 110 100 90 80 70 60 50 40 30 20 10 ℄ 10 20 30 40 50 60 70 80 90 100 110 120

3590 CHANNEL 1 X-SECTIONS STA. 3+00 TO STA. 6+00

FAY-35-2.57

328
522

Q:\3590\3590C1X2



CHANNEL CURVE DATA

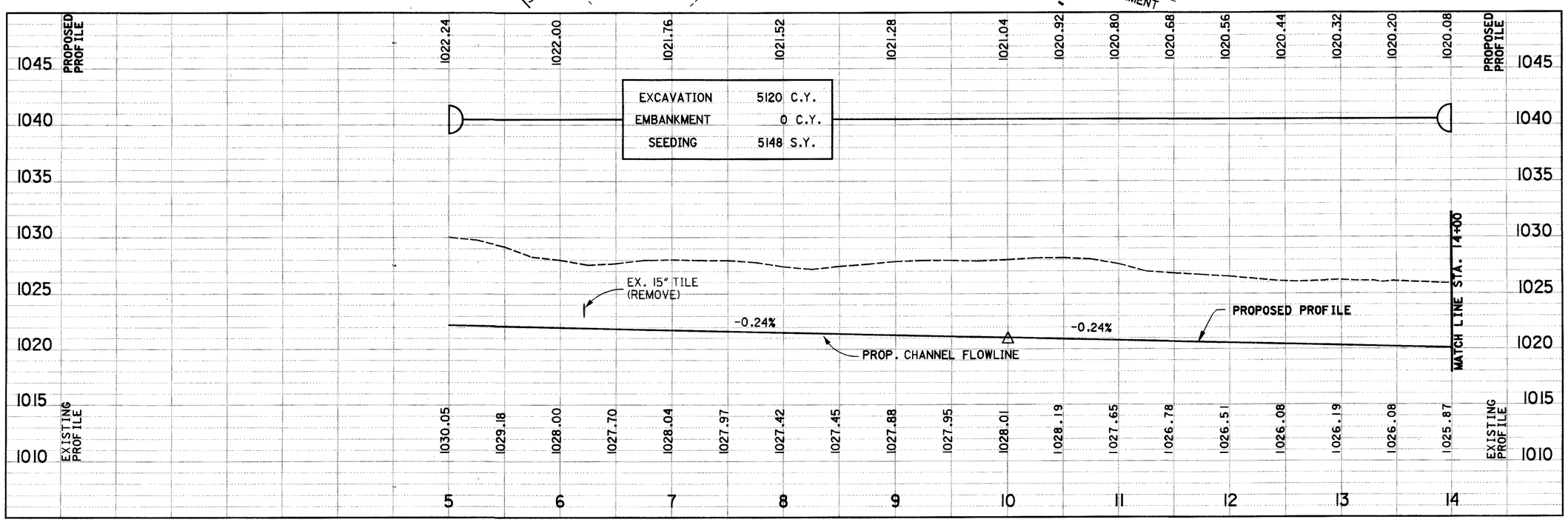
P.I. STA. 6+91.48
° = 45°-44'-45"Lt.
Dc= 12°-43'-57"
R = 450.00'
T = 189.84'
L = 359.29'
E = 38.40'

TYPICAL CHANNEL SECTION

BENCH MARK #38
SQUARE CUT ON SW WINGWALL OF BRIDGE OVER CREEK ON HARMONY RD. NORTH OF PROPOSED US 35 STA. 37+47, 15' LT. EL. 1027.92

FOR CROSS-SECTIONS, SEE SHEETS 330-332A
FOR QUANTITIES, SEE SHEETS 41,42

FOR PROPOSED USR 35, SEE SHEETS 72,73



PLAN & PROFILE CHANNEL 2

FAY-35-2.57

329
522

REVISED 1/00 BY FYC

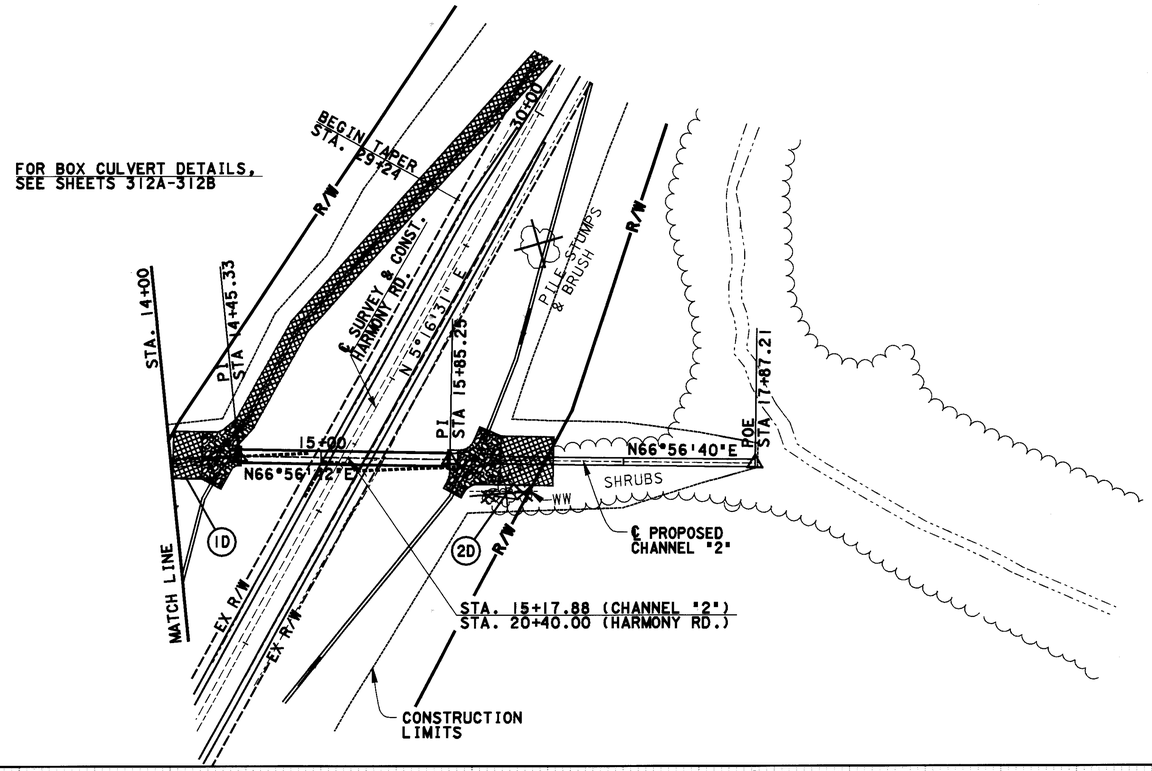
HORIZONTAL SCALE IN FEET
0 25 50 100

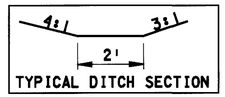
I:\pr\project\fay35\fay-35-2.57\329.dgn 01-FEB-2000 2:46PM fcheek

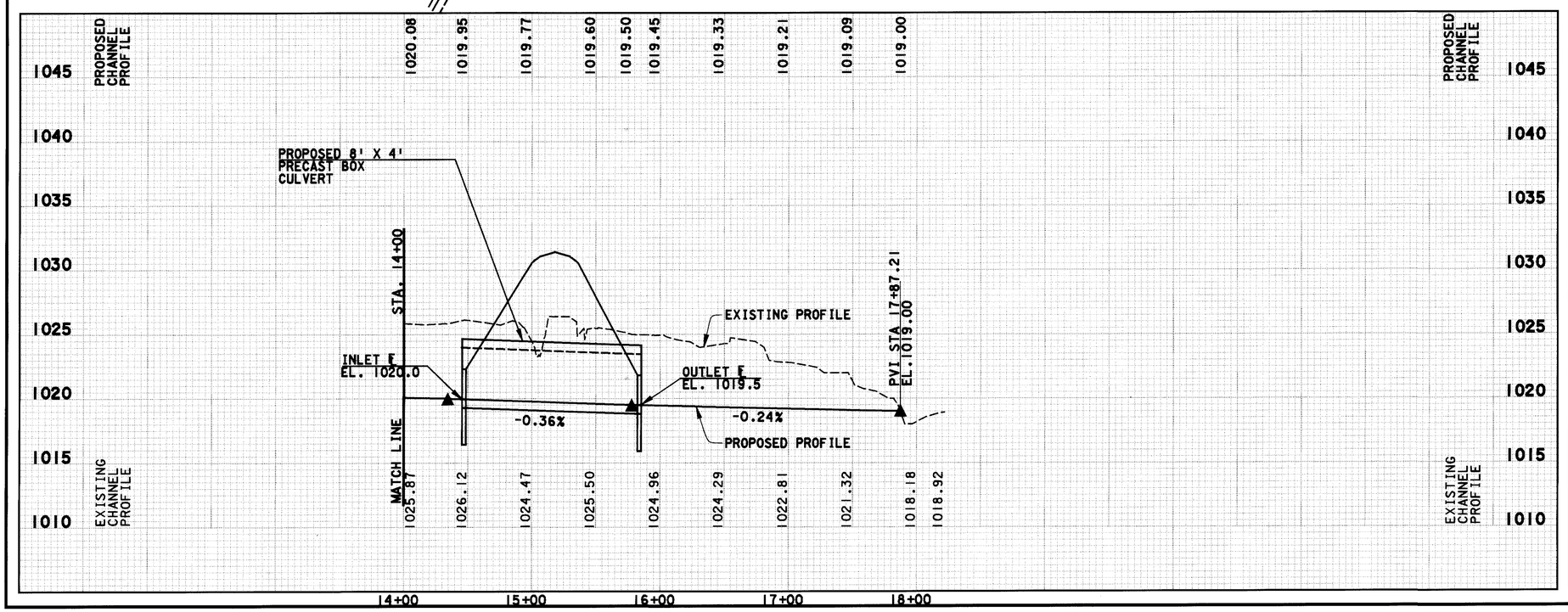
11\pr\project\fay35\fay-35-2.57\ 329.dgn

BENCH MARK #3
SQUARE CUT ON NW CORNER OF WINGWALL, WEST SIDE HARMONY RD. STA. 27+37, 11.5' LT. EL. 1025.93

BENCH MARK #38
SQUARE CUT ON SW WINGWALL OF BRIDGE OVER CREEK ON HARMONY RD. NORTH OF PROPOSED US 35
STA. 37+47, 15' LT.
EL. 1027.92







REF NO.	STATION FROM	STATION TO	SIDE	670 DITCH EROSION PROTECTION SQ. YD.
1D	14+00	14+20		66
2D	16+18	16+51		124
TOTALS CARRIED TO GENERAL SUMMARY				190

HORIZONTAL SCALE IN FEET
0 25 50 100

REVISED	CALCULATED	CHECKED
11/99	PJL	JPC
BY		
FYC		

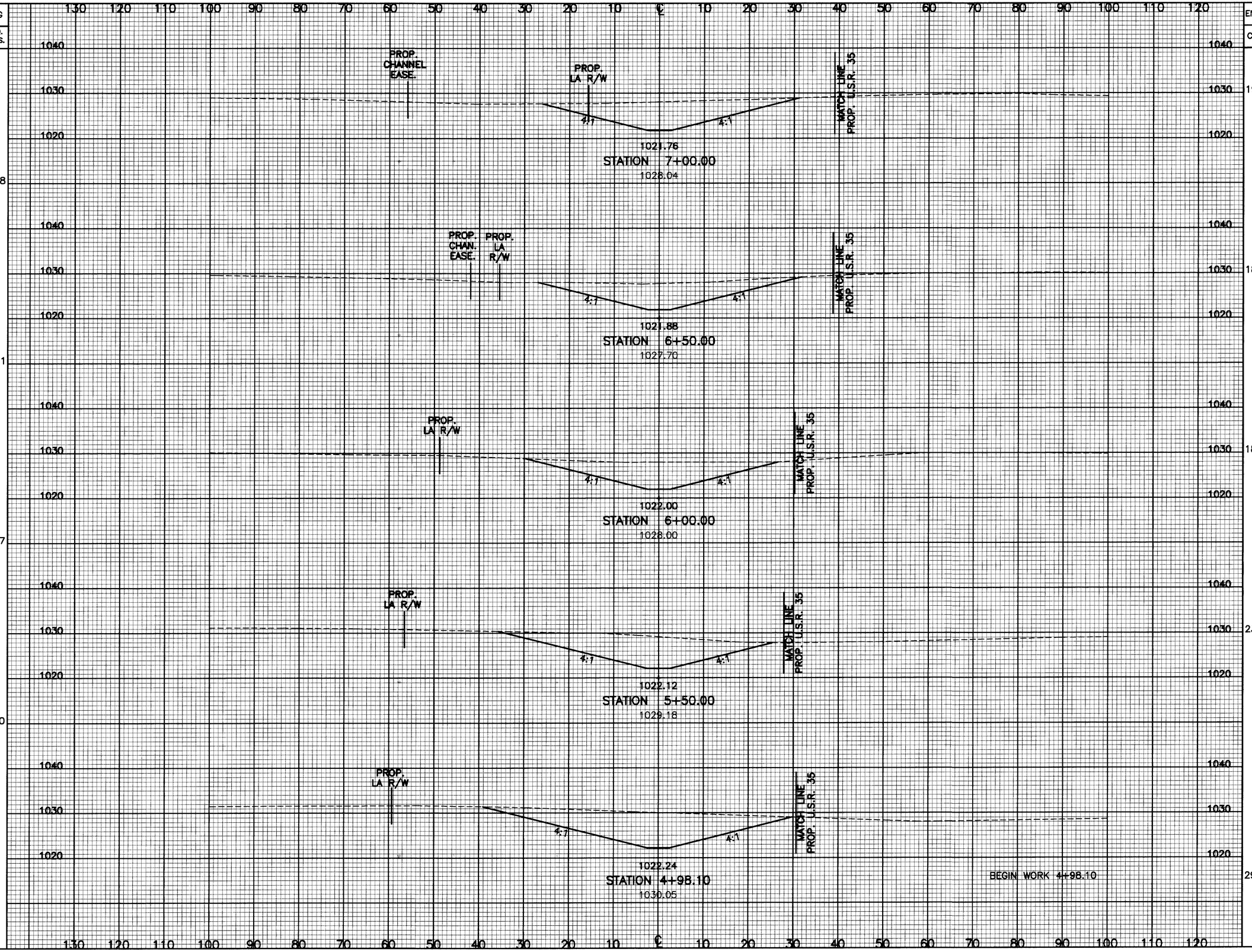
PLAN AND PROFILE - CHANNEL 2
STA. 14+00 TO STA. 17+87.21

FAY-35-2.57

329A
522

I:\pr\project\fay35\fay-35-2.57\ 06912dfr.dgn

SEEDING END WIDTH	SEEDING SQ. YDS.
62	308
49	261
45	267
51	300
57	



END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
195	0		
		354	0
187	0		
		342	0
182	0		
		389	0
238	0		
		514	0
296	0		

CALCULATED J.L.G.
CHECKED W.G.Y.

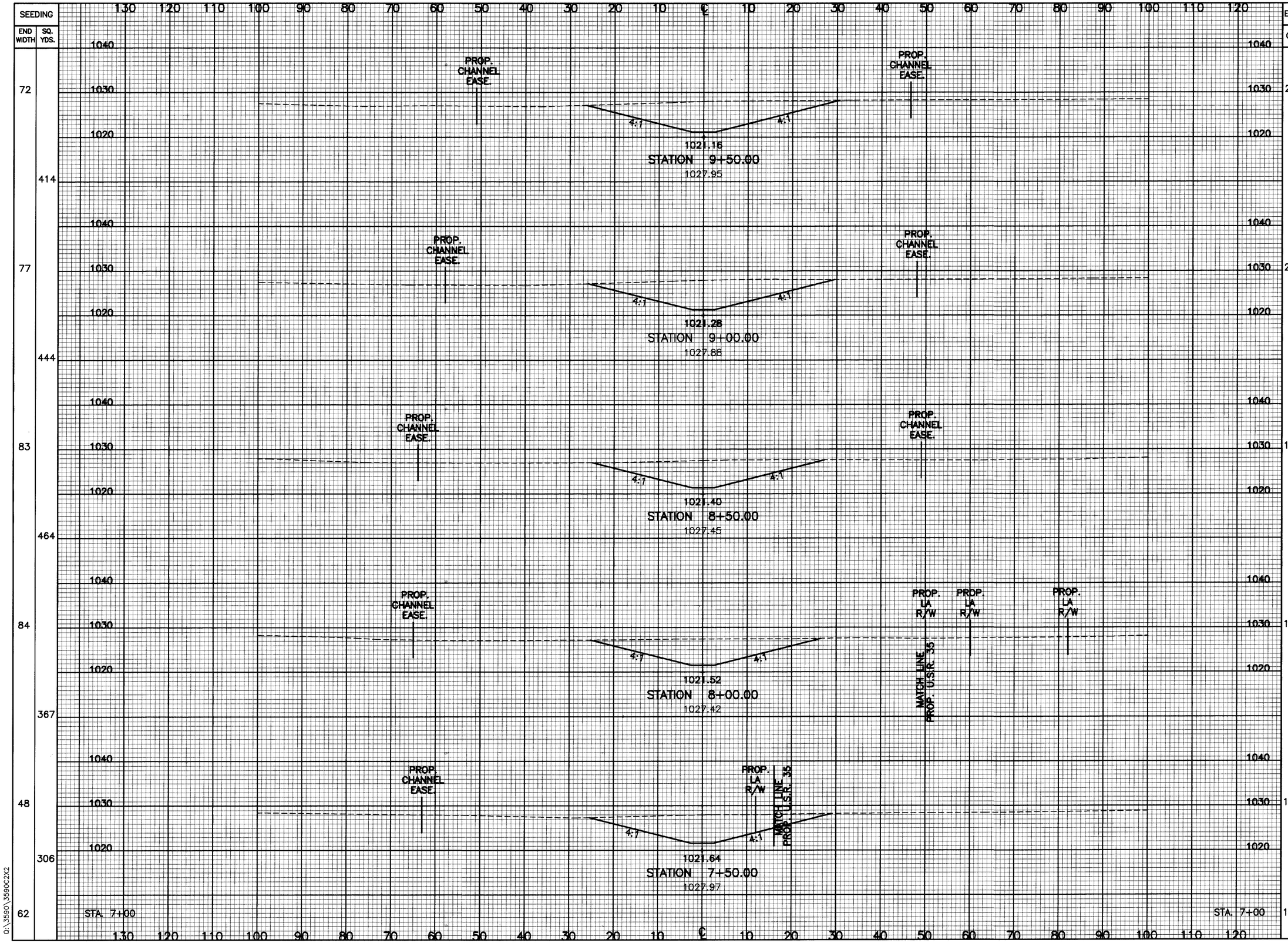
3590 CHANNEL 2 X-SECTIONS STA. 5+00 TO STA. 7+00

FAY-35-2.57

330
522

Q:\3590\3590C2X1

SEEDING		END AREA		VOLUME		CALCULATED J.L.G. CHECKED W.G.Y.
END WIDTH	SQ. YDS.	CUT	FILL	CUT	FILL	
72	414	209	0	382	0	
77	444	203	0	350	0	
83	464	175	0	322	0	
84	367	172	0	336	0	
48	306	190	0	357	0	
62		195	0			



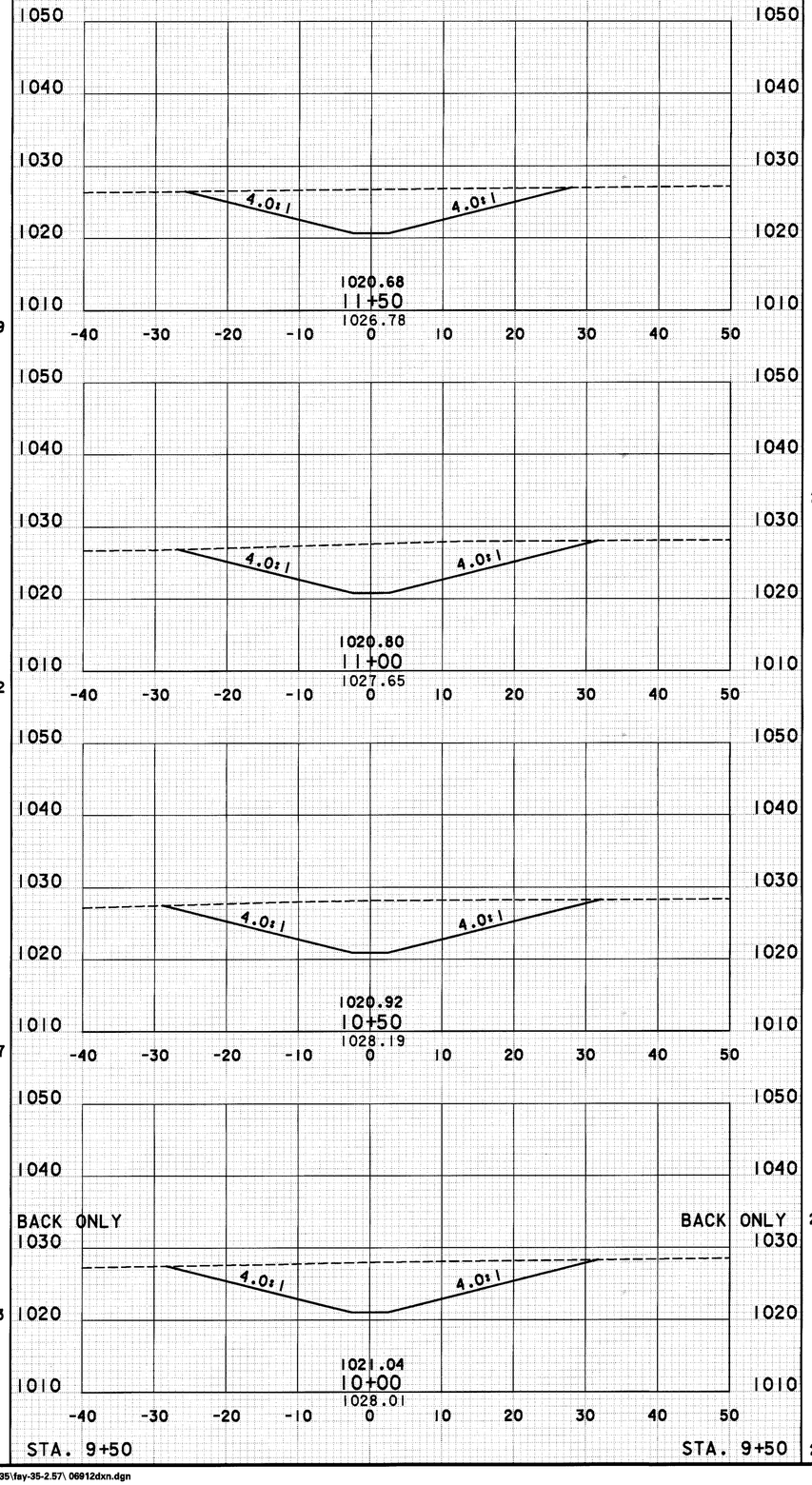
3590 CHANNEL 2 X-SECTIONS STA. 7+50 TO STA. 9+50

FAY-35-2.57

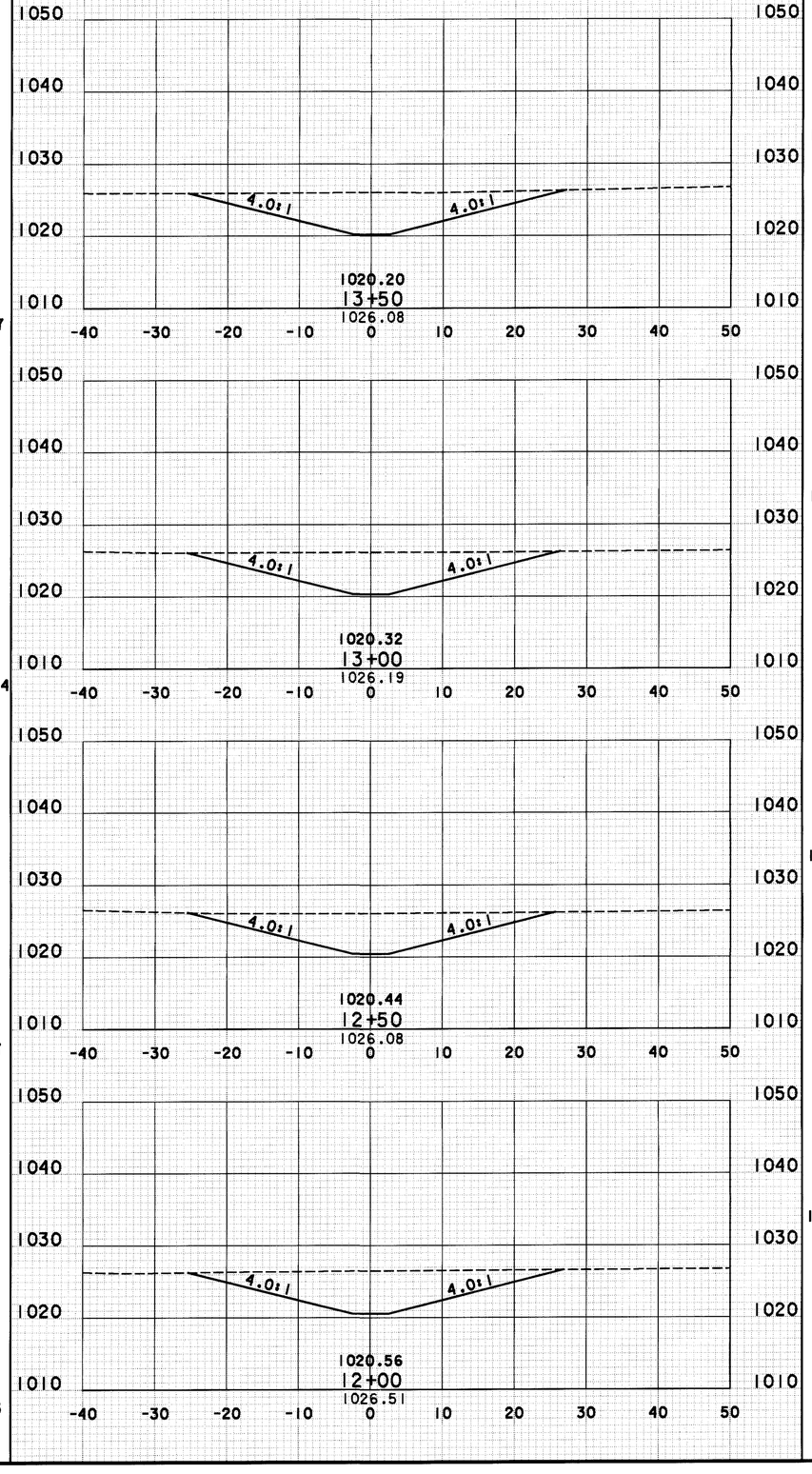
331
522

Q:\3590\3590C2X2

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
55	319	183	0	368	0
60	342	214	0	420	0
63	347	239	0	428	0
66	383	228	0	405	0
72		209	0		



SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
54	297	168	0	309	0
53	294	166	0	298	0
53	297	156	0	302	0
54	303	170	0	327	



REVISED 1/00 BY FYC

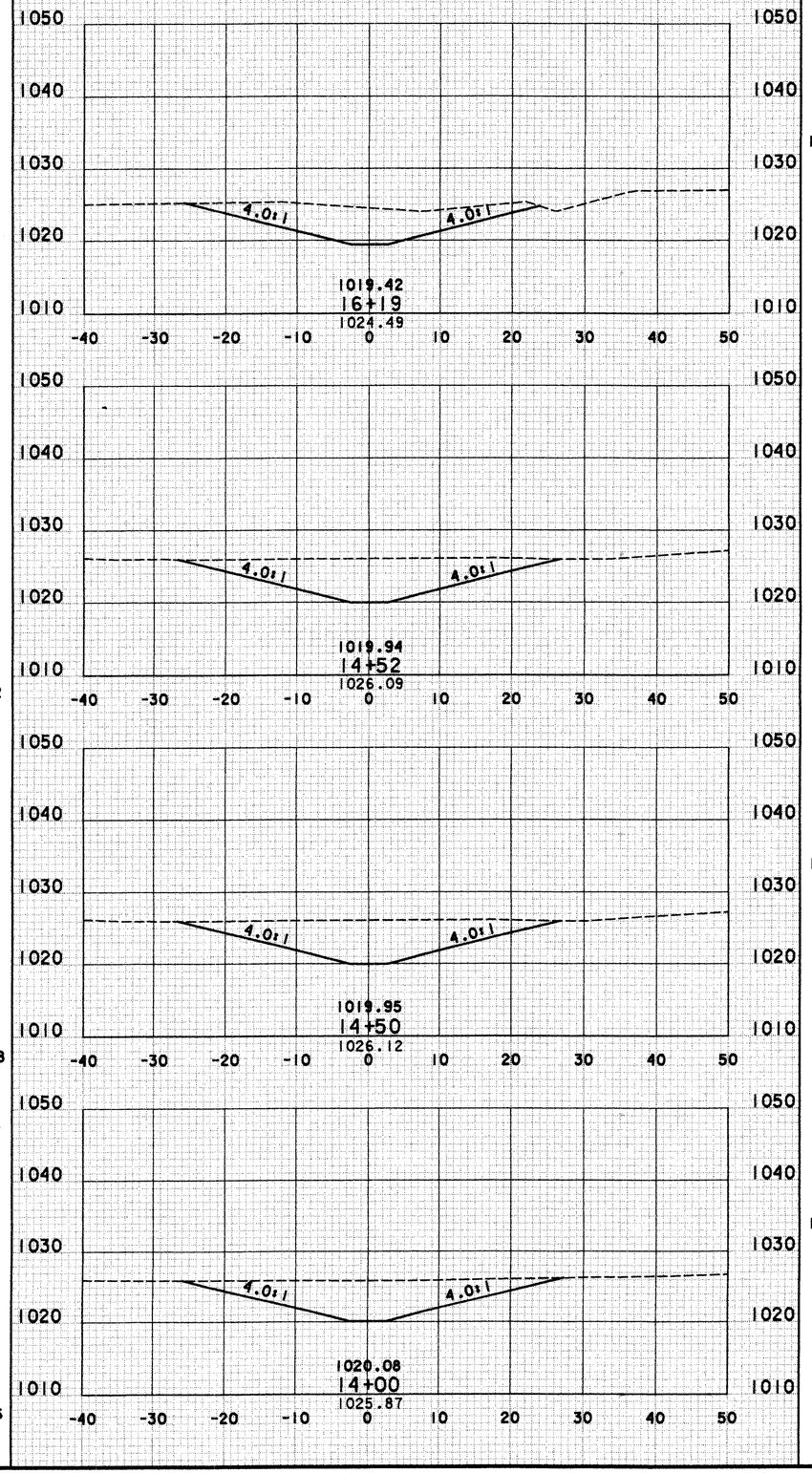
3590 CHANNEL 2 - CROSS SECTIONS
STA. 10+00 TO STA. 13+50

FAY-35-2.57

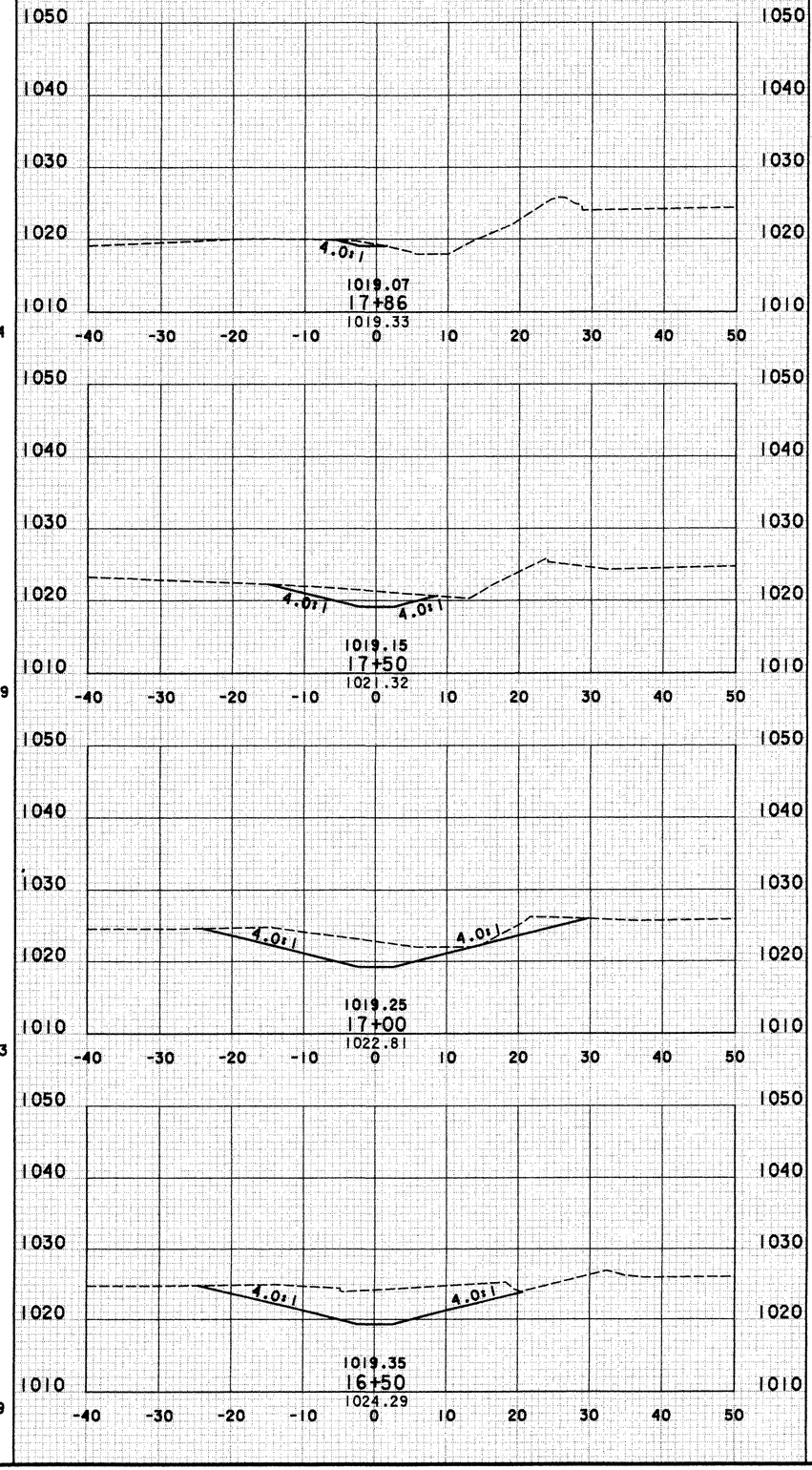
332
522

I:\pr\project\fay35\fay-35-2.57\ 06912dxn.dgn

SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
51		149	0		
55	12	181		14	0
55	308	182	0	331	0
56		175	0		
	306			318	0



SEEDING END WIDTH	SEEDING SQ. YDS.	END AREA CUT	END AREA FILL	VOLUME CUT	VOLUME FILL
3		7	0		
54				27	0
24		34	0		
219				123	0
55		99	0		
283				222	0
47		141	0		
169				166	0



CALCULATED

CHECKED

3590 CHANNEL 2 - CROSS SECTIONS
STA. 14+00 TO STA. 17+86

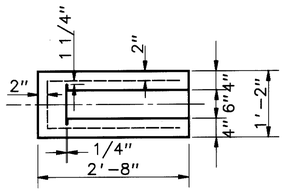
FAY-35-2.57

332A
522

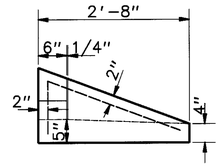
I:\pr\project\fay35\fay-35-2.57\ 06912dxn.dgn

ITEM 604–PRECAST REINFORCED CONCRETE OUTLET

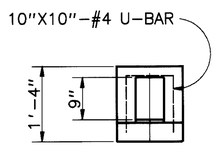
THE CONCRETE OUTLET SHALL MEET THE REQUIREMENTS OF ITEM 604 IN THE CONSTRUCTION & MATERIALS SPECIFICATIONS. PAYMENT SHALL BE MADE ON AN EACH BASIS. PAYMENT SHALL INCLUDE THE COST OF THE SOD & WIRE CLOTH.



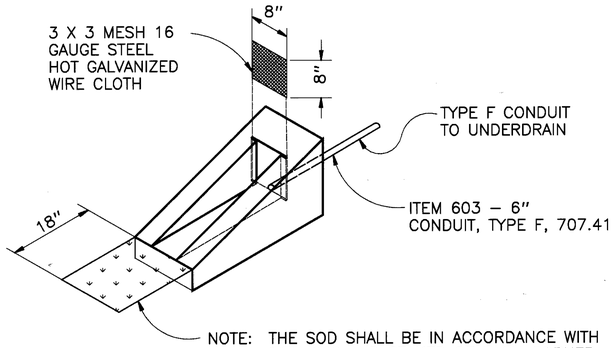
TOP VIEW



SIDE VIEW



FRONT VIEW



DESCRIPTION: THE ITEM SHALL CONSIST OF FURNISHING AND INSTALLING A PIPE UNDERDRAIN SYSTEM IN ACCORDANCE WITH THE SPECIFICATIONS, DETAILS AS SHOWN ON THE PLANS, AND AS DIRECTED BY THE ENGINEER.

MATERIALS: THE UNDERDRAIN SHALL BE A PIPE UNDERDRAIN SYSTEM PER ITEM 605.

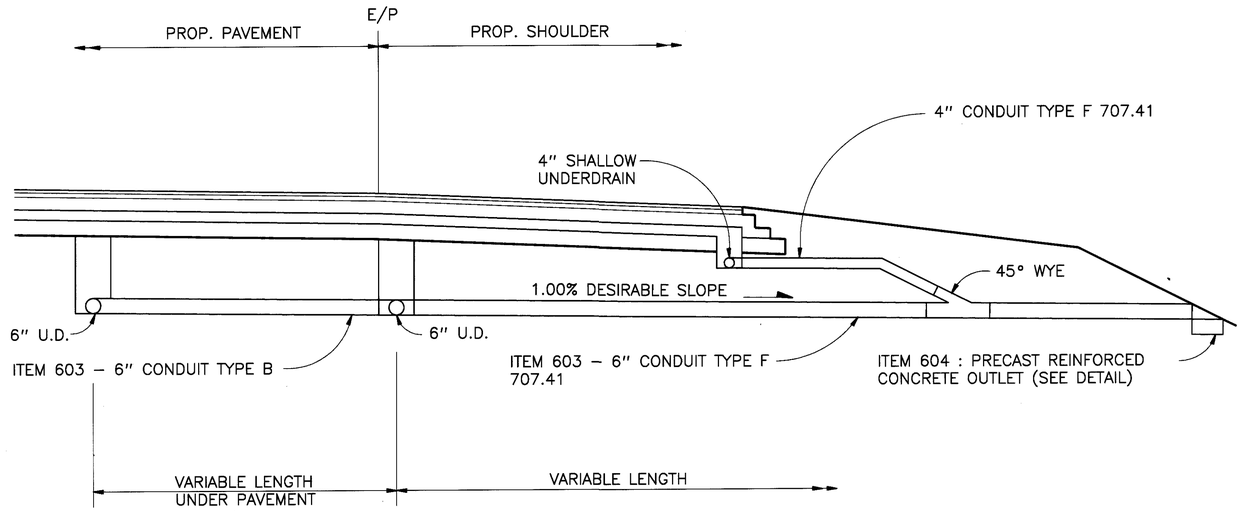
CONSTRUCTION: THE UNDERDRAIN OUTLETS SHALL BE PLACED IN ACCORDANCE WITH ITEM 603 AS DIRECTED BY THE ENGINEER, USING OUTLET FITTINGS. THE MANUFACTURER SHALL SUPPLY OUTLET FITTINGS WHICH WILL MAKE THE TRANSITION BETWEEN THE PRECAST REINFORCED CONCRETE OUTLET, OR CATCH BASIN AND THE OUTLET PIPE. FITTINGS SHALL BE INSTALLED AS RECOMMENDED BY THE MANUFACTURER.

THE OUTLETS FOR THE UNDERDRAIN SYSTEM SHALL BE CONSTRUCTED AS SOON AS POSSIBLE AFTER PLACEMENT OF THE UNDERDRAIN.

METHOD OF MEASUREMENT: COMPLETED AND ACCEPTED UNDERDRAINS WILL BE MEASURED BY THE LINEAR FOOT IN PLACE.

BASIS OF PAYMENT: WORK COMPLETED AND ACCEPTED UNDER THIS ITEM AND MEASURED WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR ITEM 605. WHICH PRICE SHALL BE FULL COMPENSATION FOR EXCAVATION AND BACKFILL; FOR FURNISHING MATERIALS, INCLUDING MATERIAL FOR SPLICES; OUTLET FITTINGS; FOR ALL LABOR, TOOLS, EQUIPMENT, AND INCIDENTALS NECESSARY TO COMPLETE THE WORK.

NOTE: THE COST OF THE 4" CONDUIT TYPE F, AND NECESSARY PIPE BENDS AND BRANCHES NEEDED TO CONNECT THE UNDERDRAINS SHALL BE INCLUDED WITH THE COST OF THE 6" CONDUIT TYPE F.

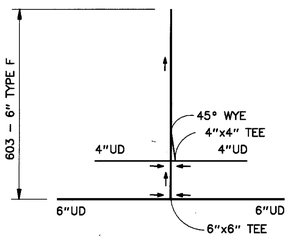


NOTE: FOR UNDERDRAIN OUTLETS INTO CATCH BASINS, 6" CONDUIT TYPE F, SHALL BE USED ENTIRELY BETWEEN THE UNDERDRAIN AND THE CATCH BASIN.

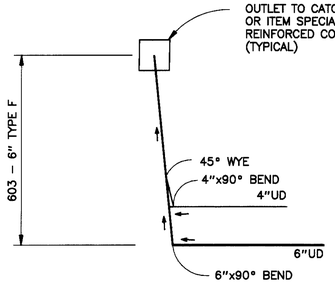
CALCULATED | CHECKED

UNDERDRAIN OUTLET DETAILS

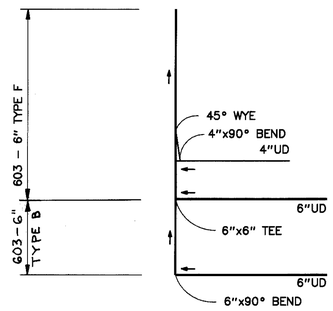
FAY–35–2.57



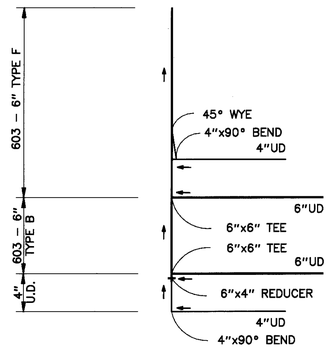
TYPE 1



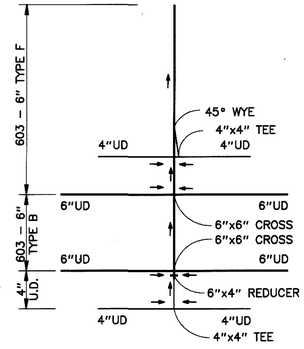
TYPE 2



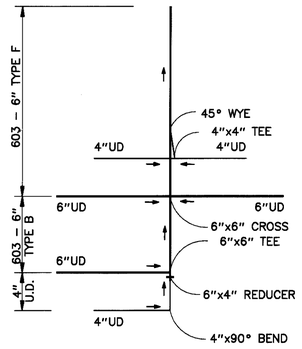
TYPE 3



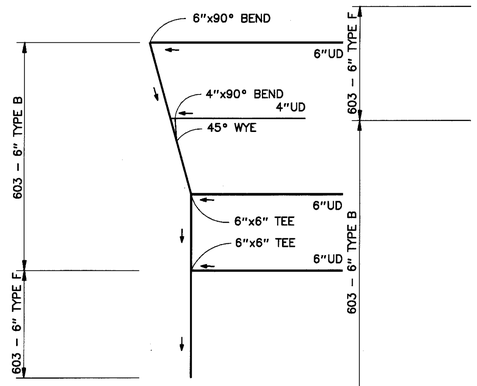
TYPE 4



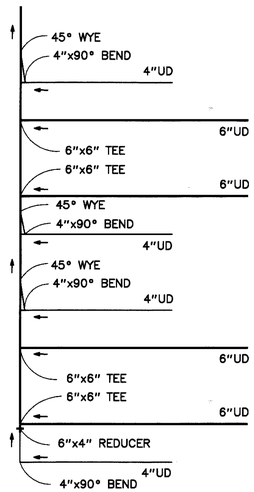
TYPE 5



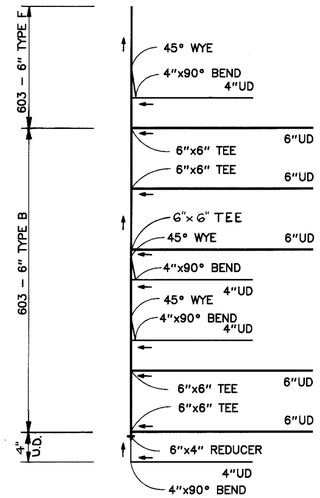
TYPE 6



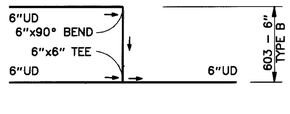
TYPE 7



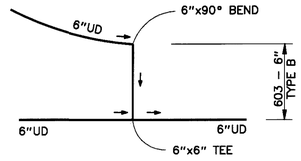
TYPE 8



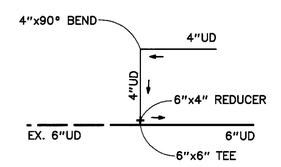
TYPE 9



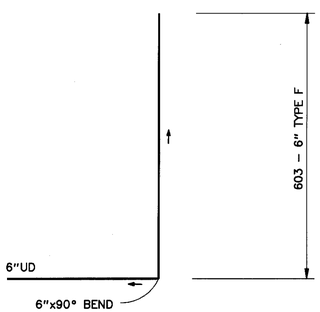
TYPE 10



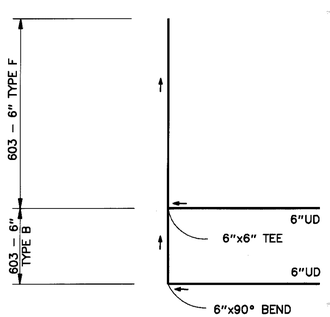
TYPE 11



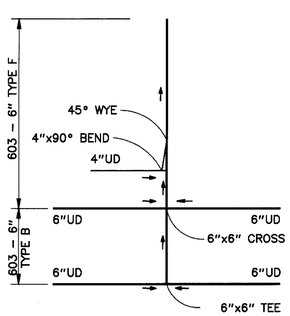
TYPE 12



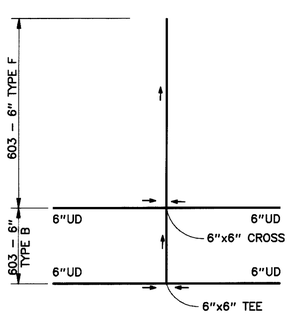
TYPE 13



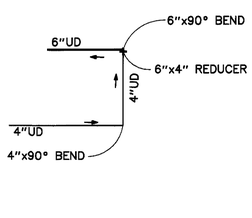
TYPE 14



TYPE 15



TYPE 16



TYPE 17

CALCULATED | CHECKED

REVISED 10-99 BY GFR

UNDERDRAIN OUTLET DETAILS

FAY-35-2.57

334
522

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE		4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
1-UD	135+74 - 136+80.02			106																			
2-UD	135+74 - 136+85.79				112																		
3-UD	135+74 - 136+99.65				126																		
4-UD	135+74 - 137+03.11			130																			
5-UD	135+74	4		6				37		24				1			2		1	1		2	
6-UD	135+74 - 137+30.83			157																			
7-UD	135+74 - 137+34.29				161																		
8-UD	135+74 - 137+48.15				175																		
9-UD	135+74 - 137+53.92			180																			
10-UD	135+74	4		6				37		24				1			2		1	1		2	
11-UD	138+50.85 - 141+20			270																			
12-UD	138+51.85 - 141+20				269																		
13-UD	138+65.71 - 141+20				245			10															
14-UD	138+69.17 - 141+20			251																			
15-UD	138+50.85 - 138+69.17	4		7				23		28				1			2		1	1		2	
16-UD	139+02.50 - 141+20			218																			
17-UD	139+00.35 - 141+20				220																		
18-UD	139+14.21 - 141+20				206																		
19-UD	139+19.98 - 141+20			200																			
20-UD	139+10 - 139+19.98	4		12				30		28			CB-1	-			2		1	1		2	
21-UD	141+25 - 147+20			595																			
22-UD	141+25 - 147+20				595																		
23-UD	141+25 - 147+20				595																		
24-UD	141+25 - 147+20			595																			
25-UD	141+25	4		10				30		24			CB-2	-			2		1	1		2	
26-UD	141+29 - 147+20			591																			
27-UD	141+30 - 147+20				540		50																
28-UD	141+30 - 147+20				590																		
29-UD	141+30 - 147+20			590																			
30-UD	141+25 -141+30	4		10				30		24			CB-2	-			2		1	1		2	
31-UD	147+25 - 150+20			295																			
32-UD	147+25 - 150+20					295																	
33-UD	147+25 - 150+20					245	50																
34-UD	147+25 - 150+20			295																			
35-UD	147+25 - 150+20			295																			
36-UD	147+25 - 150+20					245	50																
37-UD	147+25 - 150+20					245	50																
38-UD	147+25 - 150+20			295																			
39-UD	147+25	8		10				35		108			CB-9	-			4		1	3		4	
40-UD	150+25 - 156+20			595																			
41-UD	150+25 - 156+20					595																	
42-UD	150+25 - 156+20					545	50																
43-UD	150+25 - 156+20			595																			
44-UD	150+25	4		10				30		24			CB-4	-			2		1	1		2	
45-UD	150+29 - 156+20			591																			
46-UD	150+30 - 156+20					590																	
47-UD	150+25 - 150+30	2						30					CB-4	-			1			1	1		
48-UD	150+30 - 156+20					590																	
49-UD	150+28 - 156+20			592																			
50-UD	150+25 - 150+30	2						33					CB-10	-			1			1	1		
51-UD	156+25 - 159+20			295																			
52-UD	156+25 - 159+20					295																	
53-UD	156+25 - 159+20					245	50																
54-UD	156+25 - 159+20			295																			
55-UD	156+25	4		10				30		24			CB-5	-			2		1	1		2	
56-UD	156+29 - 159+20			291																			
57-UD	156+30 - 159+20					290																	
58-UD	156+25 - 156+30	2						30					CB-5	-			1			1	1		
59-UD	156+30 - 159+20					290																	
60-UD	156+28 - 159+20			292																			
61-UD	156+25 - 156+30	2						25					CB-12	-			1			1	1		
SUB-TOTAL TO SHEET 344				8690	3834	4470	300	410		308				3									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

335
522

I:\pr\project\fay35\fay-35-2.57\335.dgn 18-OCT-1999 11:29AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
62-UD	159+25 - 165+20		595																			
63-UD	159+25 - 165+20				595																	
64-UD	159+25 - 165+20				545	50																
65-UD	159+25 - 165+20		595																			
66-UD	159+25 - 165+20		595																			
67-UD	159+25 - 165+20				545	50																
68-UD	159+25 - 165+20				545	50																
69-UD	159+25 - 165+20		595																			
70-UD	159+25	8	10				25		108			CB-13	-			4		1	3		4	
71-UD	165+25 - 167+95		270																			
72-UD	165+25 - 167+95				270																	
73-UD	165+25 - 167+95				220	50																
74-UD	165+25 - 167+95		270																			
75-UD	165+25	4	10				30		24			CB-6	-			2		1	1		2	
76-UD	165+29 - 167+95		266																			
77-UD	165+30 - 167+95				265																	
78-UD	165+25 - 165+30	2					30					CB-6	-			1			1	1		
79-UD	165+30 - 167+95				265																	
80-UD	165+28 - 167+95		267																			
81-UD	165+25 - 165+30	2					25					CB-15	-			1			1	1		
82-UD	168+00 - 173+95		596																			
83-UD	168+00 - 173+95				596																	
84-UD	168+00 - 173+95				546	50																
85-UD	168+00 - 173+95		596																			
86-UD	168+00 - 173+95		595																			
87-UD	168+00 - 173+95				545	50																
88-UD	168+00 - 173+95				545	50																
89-UD	168+00 - 173+97		597																			
90-UD	168+00	8	10				24		108			CB-16	-			4		1	3		4	
91-UD	173+95 - 174+00	2					32					CB-18	-			1			1	1		
92-UD	174+00 - 178+45		452																			
93-UD	174+00 - 178+45			451																		
94-UD	174+00 - 178+45			448																		
95-UD	174+00 - 178+45		448																			
96-UD	174+00	4	10				30		24			CB-19	-			2		1	1		2	
97-UD	174+05 - 178+45		438																			
98-UD	174+05 - 178+45			438																		
99-UD	174+05 - 178+45			435																		
100-UD	174+03 - 178+45		436																			
101-UD	174+00 - 174+05	4	6				32		24			CB-18	-			2		1	1		2	
102-UD	178+50 - 182+45		401																			
103-UD	178+50 - 182+45			300		100																
104-UD	178+50 - 182+45			398																		
105-UD	178+50 - 182+45		397																			
106-UD	178+50	4	6				36		24				1			2		1	1		2	
107-UD	178+50 - 182+45		393																			
108-UD	178+50 - 182+45			393																		
109-UD	178+50 - 182+45			391																		
110-UD	178+50 - 182+45		390																			
111-UD	178+50	4	6				32		24				1			2		1	1		2	
112-UD	182+55 - 187+95		548																			
113-UD	182+55 - 187+95			547																		
114-UD	182+55 - 187+95			544																		
115-UD	182+54 - 187+95		544																			
116-UD	182+50 - 182+55	4	10				30		24			CB-20	-			2		1	1		2	
117-UD	182+50 - 187+95		543																			
118-UD	182+50 - 187+95			542																		
119-UD	182+50	2					30					CB-20	-			1			1	1		
120-UD	182+50 - 187+95			539																		
121-UD	182+50 - 187+95		538																			
122-UD	182+50	2					35						1			1			1	1		
SUB-TOTAL TO SHEET 344			11433	5426	5482	450	391		360				3									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\336.dgn 18-OCT-1999 11:30AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE		4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL.	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
123-UD	188+00 - 193+25.48			533																			
124-UD	188+00 - 193+25.38				432		100																
125-UD	188+00 - 193+25.15				529																		
126-UD	188+00 - 193+25.09			528																			
127-UD	188+00	4		6				24		24				1			2		1	1		2	
128-UD	188+00 - 193+24.91			523																			
129-UD	188+00 - 193+24.79				522																		
130-UD	188+00 - 193+24.33				519																		
131-UD	188+00 - 193+24.14			517																			
132-UD	188+00	4		6				22		24				1			2		1	1		2	
133-UD	196+39.27 - 202+99			669																			
134-UD	196+39.77 - 202+99				667																		
135-UD	196+40.96 - 202+99				663																		
136-UD	196+41.26 - 202+99			662																			
137-UD	196+41.00 - 202+99			655																			
138-UD	196+41.30 - 202+99				604		50																
139-UD	196+42.51 - 202+99				649																		
140-UD	196+43.01 - 200+51.47			403									CB-47										
140A-UD	200+95 - 202+99			202																			
141-UD	202+99	8		10				22		108				1			4		1	3		4	
142-UD	203+04 - 208+95			600																			
143-UD	203+04 - 208+95				598																		
144-UD	203+04 - 208+95				595																		
145-UD	203+04 - 208+96			595																			
146-UD	208+95 - 209+00	4		10				30		24			CB-21	-			2		1	1		2	
147-UD	203+04 - 209+00			593																			
148-UD	203+04 - 209+00				493		100																
149-UD	203+04 - 209+00				589																		
150-UD	203+04 - 209+00			588																			
151-UD	209+00	4		10				30		24			CB-21	-			2		1	1		2	
152-UD	209+05 - 214+00			502																			
153-UD	209+05 - 214+00				501																		
154-UD	209+05 - 214+00				499																		
155-UD	209+05 - 214+00			498																			
156-UD	209+05 - 214+00			493																			
157-UD	209+05 - 214+00				442		50																
158-UD	209+05 - 214+00				490																		
159-UD	209+05 - 214+00			488																			
160-UD	214+00	8		10				33		108				1			4		1	3		4	
161-UD	214+05 - 218+95			494																			
162-UD	214+05 - 218+95				493																		
163-UD	214+05 - 218+95				442		50																
164-UD	214+05 - 218+96			493																			
165-UD	218+95 - 219+00	4		10				30		24			CB-22	-			2		1	1		2	
166-UD	214+05 - 219+00			494																			
167-UD	214+05 - 219+00				444		50																
168-UD	214+05 - 219+00				493																		
169-UD	214+05 - 219+00			492																			
170-UD	219+00	4		10				30		24			CB-22	-			2		1	1		2	
171-UD	219+05 - 224+00			495																			
172-UD	219+05 - 224+00				445		50																
173-UD	219+05 - 224+00				495																		
174-UD	219+05 - 224+00			495																			
175-UD	224+00	4		6				34		24				1			2		1	1		2	
176-UD	219+05 - 224+00			495																			
177-UD	219+05 - 224+00				495																		
178-UD	219+05 - 224+00				445		50																
179-UD	219+05 - 224+00			495																			
180-UD	224+00	4		6				34		24				1			2		1	1		2	
SUB-TOTAL TO SHEET 344				13086	12544		500	289		408				6									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\337.dgn 18-OCT-1999 11:31AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
181-UD	224+05 - 228+55		450																			
182-UD	224+05 - 228+55			440			10															
183-UD	224+05 - 228+55			390		50	10															
184-UD	224+05 - 228+56		451																			
185-UD	228+55 - 228+60	4	10				30		24			CB-23	-			2		1	1		2	
186-UD	224+05 - 228+60		455																			
187-UD	224+05 - 228+60			445			10															
188-UD	228+60	2					30					CB-23	-			1			1	1		
189-UD	224+05 - 227+50			335			10															
190-UD	224+05 - 227+50		345																			
191-UD	227+50	1					32						1				1		1		1	
192-UD	227+50 - 236+00			859																		
193-UD	227+50 - 236+00		861																			
194-UD	228+65 - 235+95		720																			
195-UD	228+65 - 235+95			721																		
196-UD	228+65 - 235+95			725																		
197-UD	228+65 - 235+95		726																			
198-UD	235+95	4	6				42		24				1			2		1	1		2	
199-UD	228+65 - 236+00		739																			
200-UD	228+65 - 236+00			740																		
201-UD	236+00	5	10				30		24			CB-25	-				2	1	1			2
202-UD	236+00 - 238+45		247																			
203-UD	236+00 - 238+45			247																		
204-UD	236+00 - 238+45			249																		
205-UD	236+00 - 238+45		249																			
206-UD	236+05 - 238+45		236																			
207-UD	236+05 - 238+45			237																		
208-UD	236+05 - 238+45			239																		
209-UD	236+05 - 238+45		239																			
210-UD	236+05	4	6				42		24				1			2		1	1		2	
211-UD	238+50 - 248+34.68	17	983													1		1		1		
212-UD	238+50 - 251+00			1233																		
213-UD	238+50 - 251+00			1141		100																
214-UD	238+50 - 251+00		1243																			
215-UD	238+50 - 251+00		1258																			
216-UD	238+50 - 251+00			1260																		
217-UD	238+50 - 245+67			727																		
218-UD	238+50 - 241+21.11		276																			
219-UD	238+50	8	10				44		108				1			4		1	3		4	
221-UD	C240+21.69 - 246+00			584																		
222-UD	C240+21.69 - 251+00			1095																		
223-UD	C240+21.69 - 251+00		1096																			
224-UD	C240+21.69	15					24		16				1			1			1		1	1
225-UD	D249+50 - 251+00		153																			
226-UD	D247+03 - 251+00			401																		
227-UD	D247+03 - D248+47.74			146																		
229-UD	D247+03	16					32		16				1								1	1
230-UD	251+00	9	8				31		140				1			4		1	3		5	
231-UD	251+05 - 254+50		339																			
232-UD	251+05 - 254+50			340																		
233-UD	251+05 - 254+50			341																		
234-UD	254+50	3					31		12				1			1			1	1	1	
235-UD	251+05 - 254+50			343																		
236-UD	251+05 - 254+50		343																			
237-UD	254+50	2					30					CB-26	-			1			1	1		
238-UD	251+05 - 254+46		343																			
239-UD	251+05 - 254+45			343																		
240-UD	251+05 - 254+45			346																		
241-UD	251+05 - 254+45		347																			
242-UD	254+45 - 254+50	4	8				30		32			CB-26	-			2		1	1		2	
SUB-TOTAL TO SHEET 344			12157	13927		150	468		420				8									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\338.dgn 18-OCT-1999 11:32AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
243-UD	254+55 - 258+50		389																			
244-UD	254+55 - 258+50			391																		
245-UD	254+55 - 258+50			392																		
246-UD	254+55 - 258+50		393																			
247-UD	258+50	4	6				37		24				1			2		1	1		2	
248-UD	254+55 - 258+50		398																			
249-UD	254+55 - 258+50			398																		
250-UD	254+55 - 258+50			301		100																
251-UD	254+55 - 258+50		402																			
252-UD	258+50	4	6				39		24				1			2		1	1		2	
253-UD	258+55 - 262+50		389																			
254-UD	258+55 - 262+50			390																		
255-UD	262+50	2					33						1			1			1	1		
256-UD	258+55 - 262+50			392																		
257-UD	258+55 - 262+50		393																			
258-UD	262+50	2					30					CB-27	-			1			1	1		
259-UD	258+55 - 262+46		394																			
260-UD	258+55 - 262+45			393																		
261-UD	258+55 - 262+45			396																		
262-UD	258+55 - 262+45		397																			
263-UD	262+45 - 262+50	4	10				30		24			CB-27	-			2		1	1		2	
264-UD	262+55 - 267+50		487																			
265-UD	262+55 - 267+50			479			10															
266-UD	262+55 - 267+50			432		50	10															
267-UD	262+55 - 267+50		492																			
268-UD	262+55 - 267+50		498																			
269-UD	262+55 - 267+50			489			10															
270-UD	262+55 - 267+50			492			10															
271-UD	262+55 - 267+50		504																			
272-UD	267+50	8	10				34		108				1			4		1	3		4	
273-UD	267+55 - 270+60		301																			
274-UD	267+55 - 270+60			302																		
275-UD	267+55 - 270+60			303																		
276-UD	267+55 - 270+60		304																			
277-UD	270+60	4	6				34		24				1			2		1	1		2	
278-UD	267+55 - 270+60		307																			
279-UD	267+55 - 270+60			308																		
280-UD	267+55 - 270+60			259		50																
281-UD	267+55 - 270+60		310																			
282-UD	270+60	4	6				37		24				1			2		1	1		2	
283-UD	270+70 - 277+00		630																			
284-UD	270+70 - 277+00			580		50																
285-UD	270+70	2					34						1			1			1	1		
286-UD	270+70 - 277+00			630																		
287-UD	270+70 - 277+00		630																			
288-UD	277+00	4	6				33		24				1			2		1	1		2	
289-UD	270+70 - 277+00		630																			
290-UD	270+70 - 277+00			630																		
291-UD	270+70 - 277+00			580		50																
292-UD	270+70 - 277+00		630																			
293-UD	277+00	4	6				33		24				1			2		1	1		2	
294-UD	277+05 - 280+61.75		357																			
295-UD	277+05 - 280+61.75			347			10															
296-UD	277+05 - 280+47.89			333			10															
297-UD	277+05 - 280+44.43		340																			
298-UD	280+40.96 - 280+61.75	4	7				36		28				1			2		1	1		2	
299-UD	277+05 - 280+10.60		306																			
300-UD	277+05 - 280+13.25			309																		
301-UD	277+05 - 279+99.39			295																		
302-UD	277+05 - 279+93.62		289																			
303-UD	279+92.46 - 280+00	4	12				33		28			CB-28	-			2		1	1		2	
SUB-TOTAL TO SHEET 344			10245	9821		300	503		332				10									

CALCULATED | CHECKED
REVISED 10-99 BY GFR

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\339.dgn 18-OCT-1999 11:33AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE		4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL.	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
304-UD	280+86.39 - 285+00			414																			
305-UD	280+80.61 - 285+00				370		50																
306-UD	280+66.75 - 285+00				384		50																
307-UD	280+63.29 - 285+00			437																			
308-UD	280+35.58 - 285+00			465																			
309-UD	280+32.11 - 285+00				418		50																
310-UD	280+18.25 - 285+00				482																		
311-UD	280+12.48 - 285+00			488																			
312-UD	285+00	8		10				40		108				1			4		1	3		4	
313-UD	285+05 - 291+52			647																			
314-UD	285+05 - 291+52				647																		
315-UD	291+52	2						37						1			1			1	1		
316-UD	285+05 - 291+52				647																		
317-UD	285+05 - 291+53			648																			
318-UD	291+52 - 291+57	2						30					CB-30	-			1			1	1		
319-UD	285+05 - 291+53			648																			
320-UD	285+05 - 291+52				647																		
321-UD	291+52 - 291+57	2						30					CB-30	-			1			1	1		
322-UD	285+05 - 291+52				647																		
323-UD	285+05 - 291+54			649																			
324-UD	291+52 - 291+57	2						30					CB-31	-			1			1	1		
325-UD	291+62 - 297+45			585																			
326-UD	291+62 - 297+45				585																		
327-UD	291+62	2						37						1			1			1	1		
328-UD	291+62 - 297+45				584																		
329-UD	291+61 - 297+45			585																			
330-UD	291+57 - 291+62	2						30					CB-30	-			1			1	1		
331-UD	291+61 - 297+60			599																			
332-UD	291+62 - 297+60				598																		
333-UD	291+57 - 291+62	2						30					CB-30	-			1			1	1		
334-UD	291+62 - 297+65				602																		
335-UD	291+60 - 297+65			603																			
336-UD	291+57 - 291+62	2						30					CB-31	-			1			1	1		
337-UD	297+65	6		6				34		24				1			1	1	1	1		1	1
338-UD	297+50 - 306+95			956																			
339-UD	297+50 - 306+95				904		50																
340-UD	297+50 - 306+95				950																		
341-UD	297+50 - 306+96			950																			
342-UD	297+50	4		6				41		24				1			2		1	1		2	
343-UD	306+95 - 307+00	4		10				30		24			CB-32	-			2		1	1		2	
344-UD	297+65 - 305+00			732																			
345-UD	297+65 - 305+00				732																		
346-UD	297+65 - 305+00				729																		
347-UD	297+65 - 305+00			727																			
348-UD	305+00	4		6				37		24				1			2		1	1		2	
349-UD	307+05 - 312+95			597																			
350-UD	307+05 - 312+95				586			10															
351-UD	307+05 - 312+95				583			10															
352-UD	307+05 - 312+96			594																			
353-UD	312+95 - 313+00	4		10				30		24			CB-33	-			2		1	1		2	
354-UD	305+05 - 309+90			483																			
355-UD	305+05 - 309+90				483																		
356-UD	305+05 - 309+90				481																		
357-UD	305+05 - 309+90			480																			
358-UD	309+90	4		6				34		24				1			2		1	1		2	
359-UD	310+10 - 313+00			289																			
360-UD	310+10 - 313+00				239		50																
361-UD	310+10 - 313+00				288																		
362-UD	310+10 - 313+00			287																			
363-UD	313+00	5		10				30		24			CB-33	-				2	1	1			2
SUB-TOTAL TO SHEET 344				12927	12586		250	550		276				7									

I:\pr\project\fay35\fay-35-2.57\340.dgn 18-OCT-1999 11:33AM fcheek

CALCULATED | CHECKED

REVISED 10-99 BY GFR

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY)							
			LIN.FT.													EACH							
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL.	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
364-UD	313+05 - 316+45		344																			
365-UD	313+05 - 316+45			344																		
366-UD	313+05 - 316+45			342																		
367-UD	313+04 - 316+45		343																			
368-UD	313+00 - 313+05	4	10				30		24			CB-33	-			2		1	1		2	
369-UD	313+00 - 316+45		344																			
370-UD	313+00 - 316+45			294		50																
371-UD	313+00 - 316+45			342																		
372-UD	313+00 - 316+45		342																			
373-UD	316+55 - 320+95		445																			
374-UD	316+55 - 320+95			395		50																
375-UD	316+55 - 320+95			443																		
376-UD	316+55 - 320+95		442																			
377-UD	316+55	4	6				37		24				1			2		1	1		2	
378-UD	316+55 - 320+95		439																			
379-UD	316+55 - 320+95			438																		
380-UD	316+55 - 320+95			436																		
381-UD	316+55 - 320+95		435																			
382-UD	316+55	4	6				35		24				1			2		1	1		2	
383-UD	321+05 - 326+00		500																			
384-UD	321+05 - 326+00			449		50																
385-UD	321+05 - 325+95			492																		
386-UD	321+04 - 325+95		493																			
387-UD	321+00 - 321+05	4	10				30		24			CB-34	-			2		1	1		2	
388-UD	321+00 - 325+95		494																			
389-UD	321+00 - 325+95			493																		
390-UD	321+00 - 325+95			492																		
391-UD	321+00 - 325+95		491																			
392-UD	321+00	4	6				36		24				1			2		1	1		2	
393-UD	326+00 - 329+95		395																			
394-UD	326+00 - 329+95			345		50																
395-UD	326+00 - 329+95			395																		
396-UD	326+00 - 329+95		395																			
397-UD	326+00	6	6				35		24				1			1	1	1	1		1	1
398-UD	326+00 - 329+95		395																			
399-UD	326+00 - 329+95			395																		
400-UD	326+00 - 329+95			395																		
401-UD	326+00 - 329+95		395																			
402-UD	326+00	4	6				34		24				1			2		1	1		2	
403-UD	330+05 - 340+96.68		1092																			
404-UD	330+05 - 340+87.53			1083																		
405-UD	330+05	2					36						1			1			1	1		
406-UD	330+05 - 340+65.58			1061																		
407-UD	330+04 - 340+60.09		1057																			
408-UD	330+00 - 330+05	2					30					CB-35	-			1			1	1		
409-UD	330+00 - 340+16.19		1017																			
410-UD	330+00 - 340+10.70			1011																		
411-UD	330+00	2					30					CB-35	-			1			1	1		
412-UD	330+00 - 339+88.75			989																		
413-UD	330+00 - 339+79.60		980																			
414-UD	330+00	2					35						1			1			1	1		
415-UD	343+15.28 - 351+95		873																			
416-UD	343+06.13 - 351+95			884																		
417-UD	342+84.18 - 351+95			858		50																
418-UD	342+78.69 - 351+96		915																			
419-UD	351+95 - 352+00	4	10				30		24			CB-36	-			2		1	1		2	
420-UD	342+34.79 - 352+00		968																			
421-UD	342+29.30 - 352+00			974																		
422-UD	342+07.35 - 352+00			999																		
423-UD	341+98.20 - 352+00		1009																			
424-UD	352+00	4	10				30		24			CB-36	-			2		1	1		2	
SUB-TOTAL TO SHEET 344			14673	14349		250	428		216				7									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

341
522

I:\pr\project\fay35\fay-35-2.57\341.dgn 18-OCT-1999 11:34AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY)						
			LIN.FT.													EACH						
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
425-UD	352+05 - 357+00		490																			
426-UD	352+05 - 357+00			491																		
427-UD	352+05 - 357+00			493																		
428-UD	352+05 - 357+00		493																			
429-UD	357+00	4	6				37		24				1			2		1	1		2	
430-UD	352+05 - 357+00		497																			
431-UD	352+05 - 357+00			498																		
432-UD	352+05 - 357+00			450		50																
433-UD	352+05 - 357+00		501																			
434-UD	357+00	4	6				37		24				1			2		1	1		2	
435-UD	357+05 - 361+95		485																			
436-UD	357+05 - 361+95			486																		
437-UD	357+05 - 361+95			488																		
438-UD	357+05 - 361+95		488																			
439-UD	361+95	4	6				36		24				1			2		1	1		2	
440-UD	357+05 - 362+00		497																			
441-UD	357+05 - 362+00			498																		
442-UD	357+05 - 362+00			500																		
443-UD	357+05 - 362+00		501																			
444-UD	362+00	4	10				30		24			CB-37	-			2		1	1		2	
445-UD	362+05 - 367+90		579																			
446-UD	362+05 - 367+90			580																		
447-UD	362+05 - 367+90			582																		
448-UD	362+05 - 367+90		583																			
449-UD	367+90	4	6				40		24				1			2		1	1		2	
450-UD	362+05 - 367+90		588																			
451-UD	362+05 - 367+90			588																		
452-UD	362+05 - 367+90			541		50																
453-UD	362+05 - 367+90		592																			
454-UD	367+90	4	6				37		24				1			2		1	1		2	
455-UD	368+05 - 371+95		386																			
456-UD	368+05 - 371+95			387																		
457-UD	368+05 - 371+95			388																		
458-UD	368+05 - 371+95		389																			
459-UD	371+95	4	6				39		24				1			2		1	1		2	
460-UD	368+05 - 372+00		397																			
461-UD	368+05 - 372+00			397																		
462-UD	368+05 - 372+00			399																		
463-UD	368+05 - 372+00		400																			
464-UD	372+00	4	10				30		24			CB-38	-			2		1	1		2	
465-UD	372+05 - 377+00		490																			
466-UD	372+05 - 377+00			491																		
467-UD	372+05 - 377+00			443		50																
468-UD	372+05 - 377+00		493																			
469-UD	372+05 - 377+00		497																			
470-UD	372+05 - 377+00			498																		
471-UD	372+05 - 377+00			500																		
472-UD	372+05 - 377+00		501																			
473-UD	377+00	8	10				37		108				1			4		1	3		4	
474-UD	377+05 - 382+20		511																			
475-UD	377+05 - 382+20			512																		
476-UD	380+09	1					36						1				1		1		1	
477-UD	377+05 - 382+20			513																		
478-UD	377+05 - 382+21		515																			
479-UD	382+20 - 382+25	2					30					CB-39	-			1			1	1		
480-UD	377+05 - 382+25		522																			
481-UD	377+05 - 382+25			472		50																
482-UD	377+05 - 382+25			524																		
483-UD	377+05 - 382+25		525																			
484-UD	382+25	4	10				30		24			CB-39	-			2		1	1		2	
SUB-TOTAL TO SHEET 344			11996	11719		200	419		324				8									

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

342
522

I:\pr\project\fay35\fay-35-2.57\342.dgn 18-OCT-1999 11:35AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.				603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
485-UD	382+30 - 389+65		735																			
486-UD	382+30 - 389+65			685		50																
487-UD	382+30 - 389+65			735																		
488-UD	382+30 - 389+65		735																			
489-UD	389+65	4	6				35		24				1			2		1	1		2	
490-UD	382+30 - 389+65		735																			
491-UD	382+30 - 389+65			735																		
492-UD	382+30 - 389+65			685		50																
493-UD	382+30 - 389+65		735																			
494-UD	389+65	4	6				36		24				1			2		1	1		2	
495-UD	389+80 - 391+13		133																			
496-UD	389+80 - 391+13			133																		
497-UD	389+80 - 391+13			83		50																
498-UD	389+80 - 391+14		134																			
499-UD	391+13 - 391+18	4	10				30		24			CB-40	-			2		1	1		2	
500-UD	389+80 - 396+20		640																			
501-UD	389+80 - 396+20			540		100																
502-UD	389+80 - 396+20			640																		
503-UD	389+80 - 396+20		640																			
504-UD	391+18	5	10				30		24			CB-40	-				2	1	1			2
505-UD	391+23 - 396+20		497																			
506-UD	391+23 - 396+20			497																		
507-UD	391+23 - 396+20			447		50																
508-UD	391+22 - 396+20		498																			
509-UD	391+18 - 391+23	4	10				30		24			CB-40	-			2		1	1		2	
	RAMP C																					
	C227+48.46 - C229+45																					
511-UD	C227+48.46 - C229+45			200																		
512-UD	C227+48.46 - C229+45			197																		
	C227+48.46 - C229+45																					
514-UD	C227+48.46	7					22		36				1			1			1	1	2	
516-UD	C229+55 - C230+10.15			57																		
517-UD	CC229+19.06 - C230+59.33	11		161									-							1	1	
519-UD	C230+10.15	10							16				-							1	1	
521-UD	CC229+06.70 - C235+50			596																		
522-UD	C229+55 - C235+50			595																		
524-UD	C235+50	14					27		22				1							1	1	
526-UD	C235+55 - C240+21.69			467																		
527-UD	C235+55 - C240+21.69			467																		
	RAMP D																					
529-UD	D237+20 - D238+00			81																		
531-UD	D238+00	13					24						1							1		
533-UD	D237+40 - D243+45			576		50																
534-UD	D238+05 - D243+45			547																		
536-UD	D243+45	14					31		16				1							1	1	
538-UD	D243+55 - D247+03			348																		
539-UD	D243+55 - D247+03			352																		
	SUB-TOTAL TO SHEET 344		5524	9824		350	265		210				6									

REVISED 10-99 BY GFR | CALCULATED | CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

343
522

I:\dr\project\fay35\fay-35-2.57\343.dgn 18-OCT-1999 11:36AM fcheek

605 UNDERDRAIN
DRAINAGE QUANTITIES

** ITEM 605 - 4" PIPE UNDERDRAIN, FABRIC WRAP, AS PER PLAN

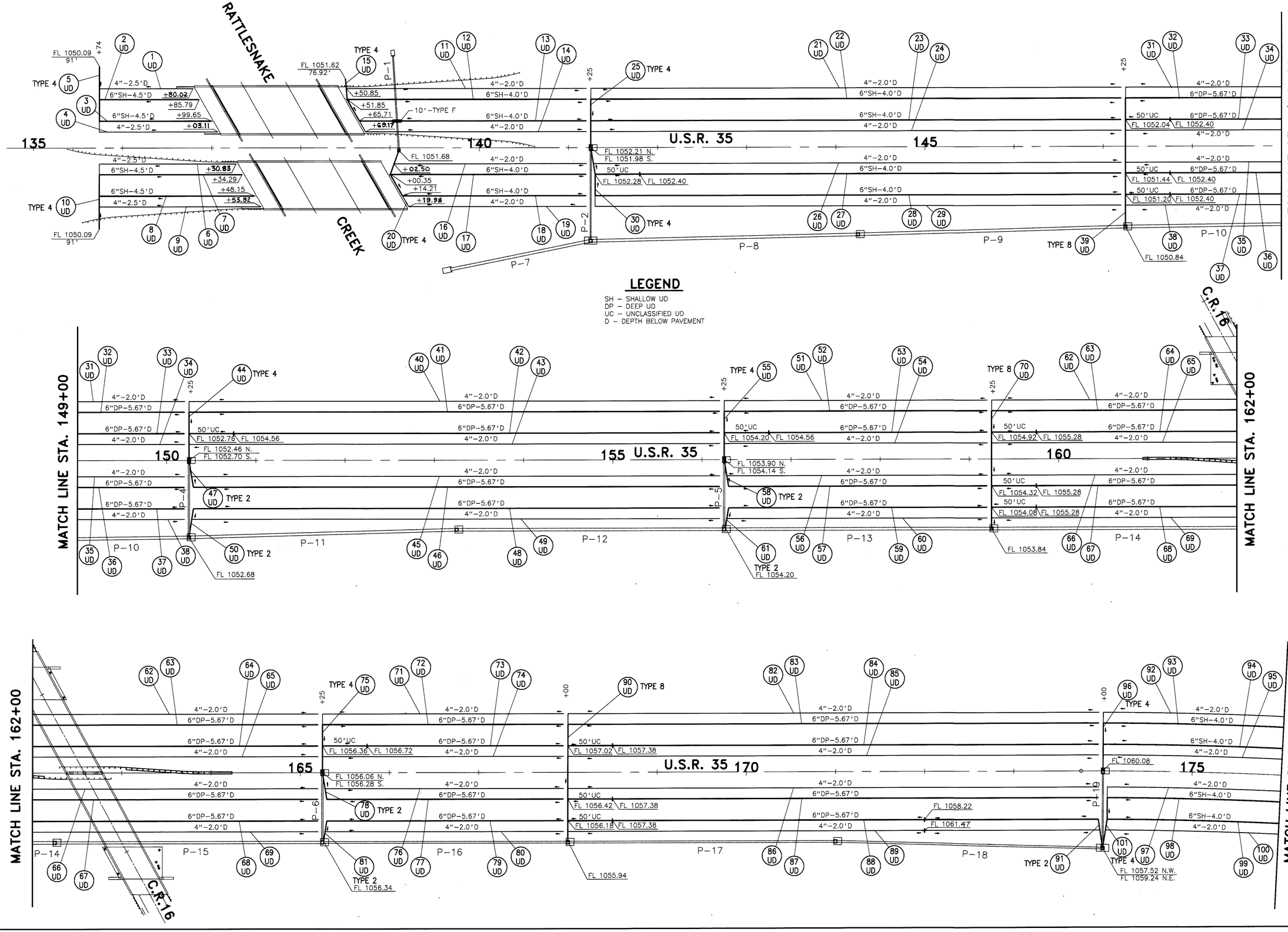
REF. NO.	STATION TO STATION	UNDERDRAIN OUTLETS	605 UNDERDRAINS LIN.FT.					603 6"CONDUIT TYPE F, 707.41		603 6"CONDUIT TYPE B			OUTLET INTO CATCH BASIN NO.	604 PRECAST REINFORCED CONCRETE OUTLET			BENDS AND BRANCHES (FOR INFORMATION ONLY) EACH						
		TYPE	4" SHALLOW	4" ** SHALLOW	6" SHALLOW	6" DEEP	6" UNCL	LIN.FT.		LIN.FT.				EACH			4"x90° BEND	4"x4" TEE	6"x4" REDUCER	6"x4" WYE	6"x90° BEND	6"x6" TEE	6"x6" CROSS
	EXIST. U.S.R. 35																						
541-UD	62+30 - 63+95	12	175											-			1		1			1	
542-UD	62+30 - 63+95				165																		
543-UD	62+30 - 63+95				165																		
544-UD	62+30 - 63+95		165																				
545-UD	62+30	2						20						1			1			1	1		
546-UD	64+08.58 - 66+45		237																				
547-UD	64+08.58 - 66+45				237																		
548-UD	64+08.58	2						25						1			1			1	1		
549-UD	64+08.58 - 66+45				237																		
550-UD	64+08.58 - 66+45		237																				
551-UD	64+08.58	2						25						1			1			1	1		
552-UD	66+50 - 68+75		225																				
553-UD	66+50 - 68+75				225																		
554-UD	66+50	2						25						1			1			1	1		
555-UD	66+50 - C227+43.46				541																		
556-UD	66+50 - C227+43.46		539																				
557-UD	66+50	2						27						1			1			1	1		
558-UD	68+80 - 75+47.80		671																				
559-UD	68+80 - 75+56.63				677																		
560-UD	68+80	2						25						1			1			1	1		
561-UD	71+97 - 75+87.97				391																		
562-UD	71+97 - 75+96.71		400																				
563-UD	78+83.25 - 85+00		617																				
564-UD	78+71.81 - 85+00				629																		
565-UD	85+00	2						26						1			1			1	1		
566-UD	79+03.25 - 83+94.22				491																		
567-UD	79+41.65 - 83+94.22		453																				
568-UD	83+94.22	2						28						1			1			1	1		
569-UD	85+05 - 89+00		395																				
570-UD	85+05 - 89+00				385			10															
571-UD	89+00	2						29						1			1			1	1		
572-UD	84+98 - 89+00				392			10															
573-UD	84+98 - 89+00		402																				
574-UD	89+00	2						24						1			1			1	1		
	MAINLINE U-TURN MEDIAN OPENING																						
575-UD	280+00 - 280+36						16	20															
576-UD	382+25 - 382+61						16	20															
SUB-TOTAL		4516		4535		32	314							10			11		1	10	10	1	
SUB-TOTAL SHEET NO.343			5524	9824		350	265		210					6			9	2	5	6	6	14	2
SUB-TOTAL SHEET NO.342			11996	11719		200	419		324					8			23	1	10	14	1	23	
SUB-TOTAL SHEET NO.341			14673	14349		250	428		216					7			21	1	9	13	4	17	1
SUB-TOTAL SHEET NO.340			12927	12586		250	550		276					7			23	4	8	18	8	15	3
SUB-TOTAL SHEET NO.339			10245	9821		300	503		332					10			25		10	15	3	22	
SUB-TOTAL SHEET NO.338			12157	13927		150	468		420					8			21	3	8	16	4	21	4
SUB-TOTAL SHEET NO.337			13086	12544		500	289		408					6			24		10	14		24	
SUB-TOTAL SHEET NO.336			11433	5426	5482	450	391		360					3			25		8	17	5	20	
SUB-TOTAL SHEET NO.335			8690	3834	4470	300	410		308					3			24		9	15	4	20	
GRAND TOTAL TO SHEET 279		4516	100731	98565	9952	2782	4037		2854					68									

REVISED 10-99 BY GFR | CALCULATED | CHECKED

UNDERDRAIN DRAINAGE QUANTITIES

FAY-35-2.57

I:\pr\project\fay35\fay-35-2.57\344.dgn 18-OCT-1999 11:37AM fcheek



LEGEND

SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT

HORIZONTAL SCALE IN FEET: 0, 25, 50, 100

REVISED 10-99 BY GFR

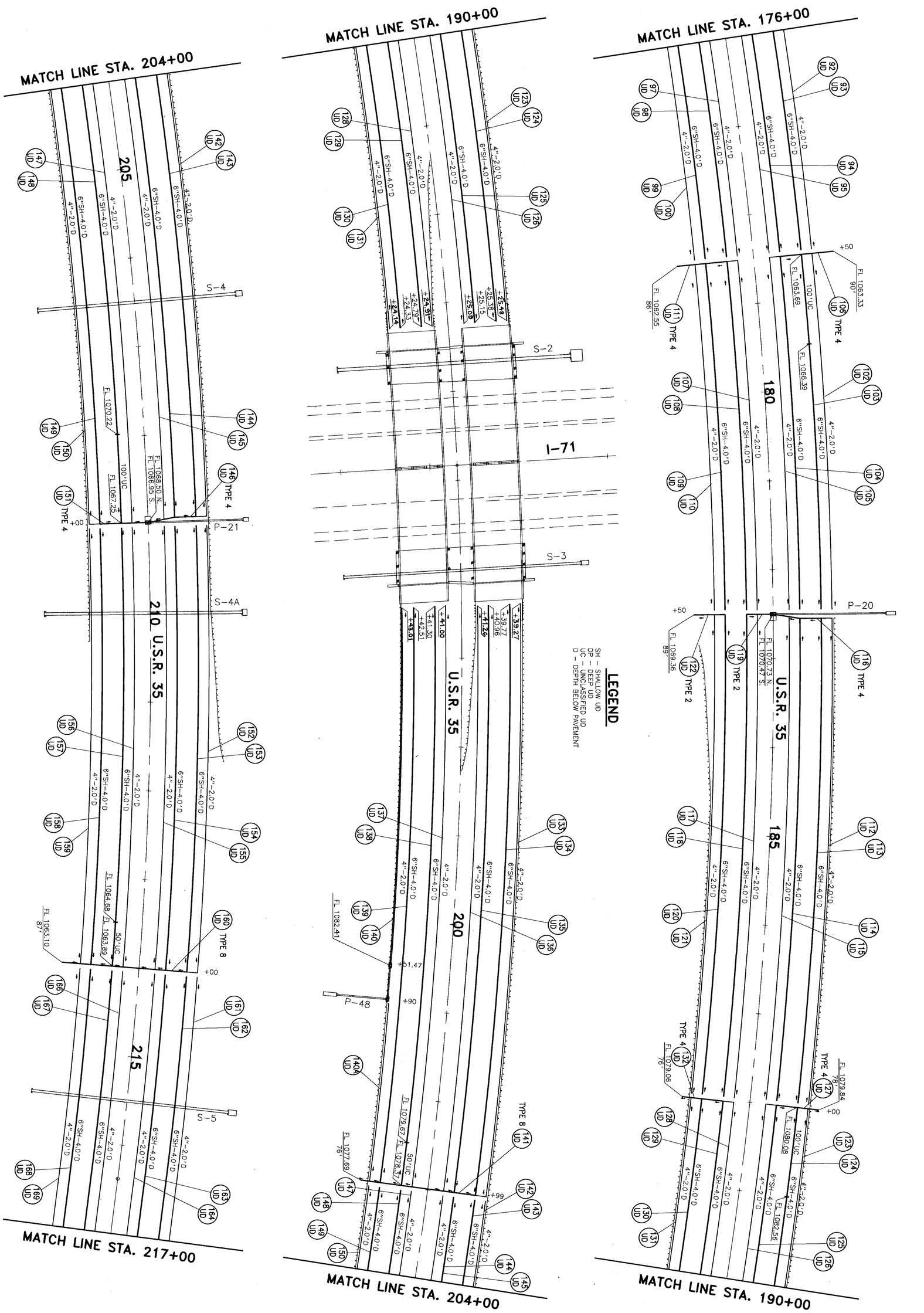
CALCULATED

CHECKED

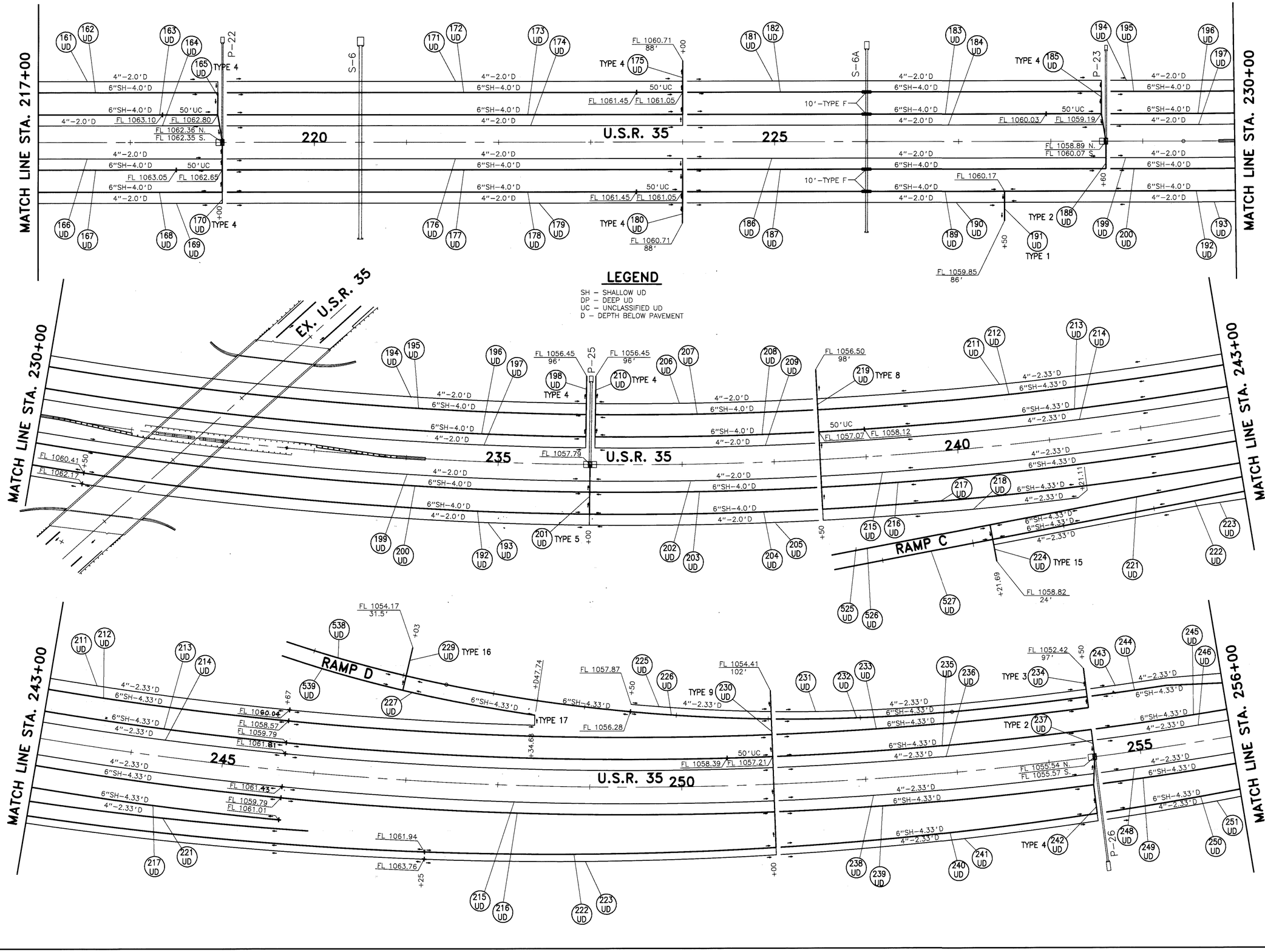
UNDERDRAINS – PROP. U.S.R. 35
STA. 135+00 TO STA. 176+00

FAY-35-2.57

345
522



346 / 522	FAY-35-2.57	UNDERDRAINS – PROP. U.S.R. 35 STA. 176+00 TO STA. 217+00	REVISED 10-99	CALCULATED	0 50	
			BY GFR	CHECKED	25 100 HORIZONTAL SCALE IN FEET	



LEGEND

SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT

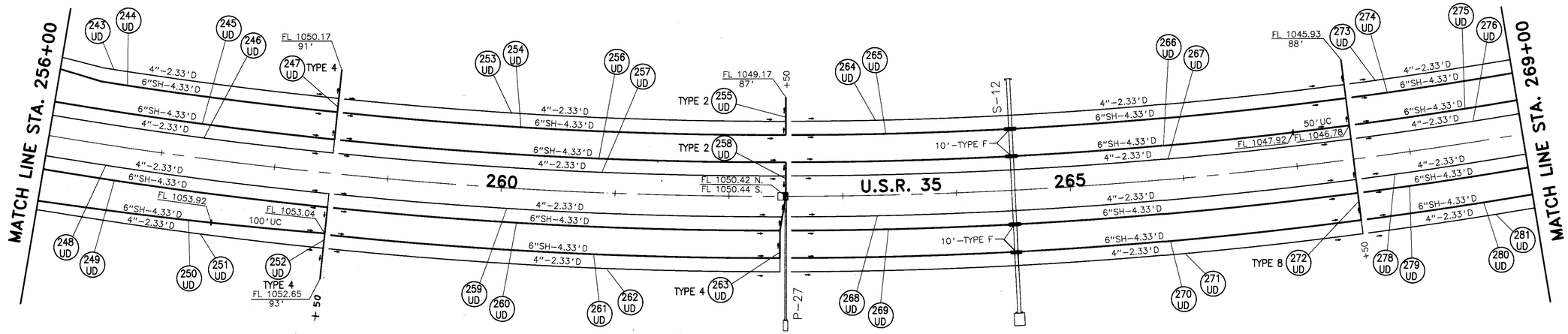
HORIZONTAL SCALE IN FEET: 0, 25, 50, 100

REVISED 10-99 BY GFR
CALCULATED
CHECKED

UNDERDRAINS STA. 217+00 TO STA. 256+00

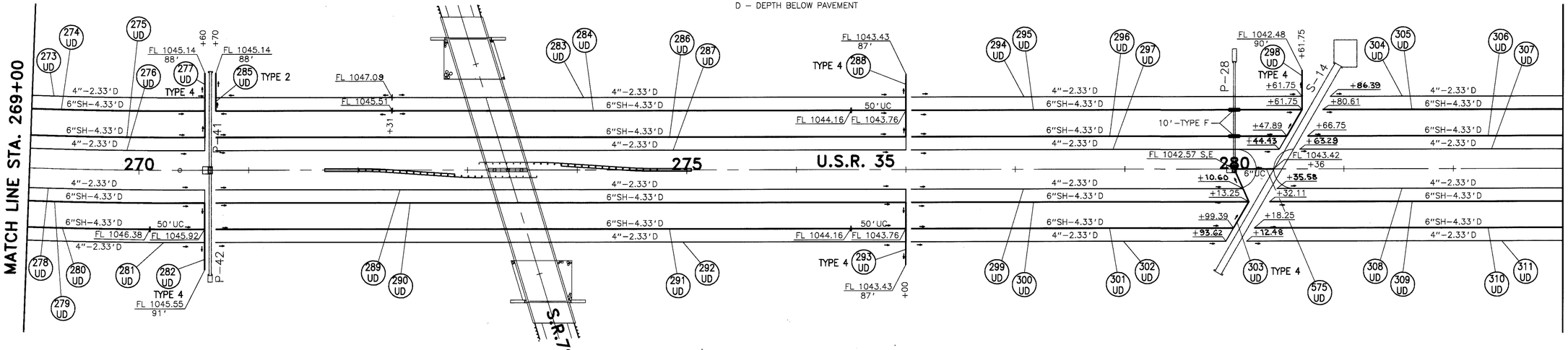
FAY-35-2.57

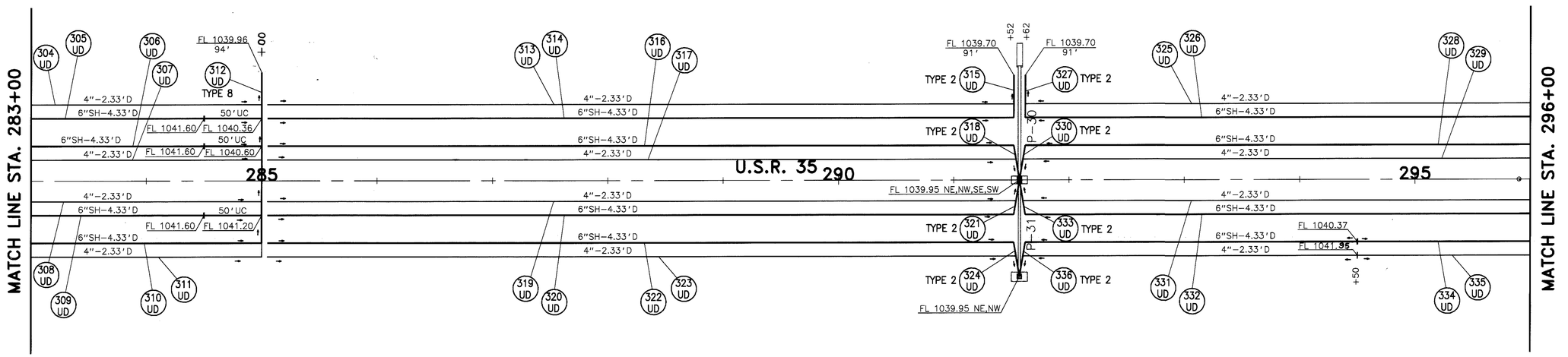
347
522



LEGEND

SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT





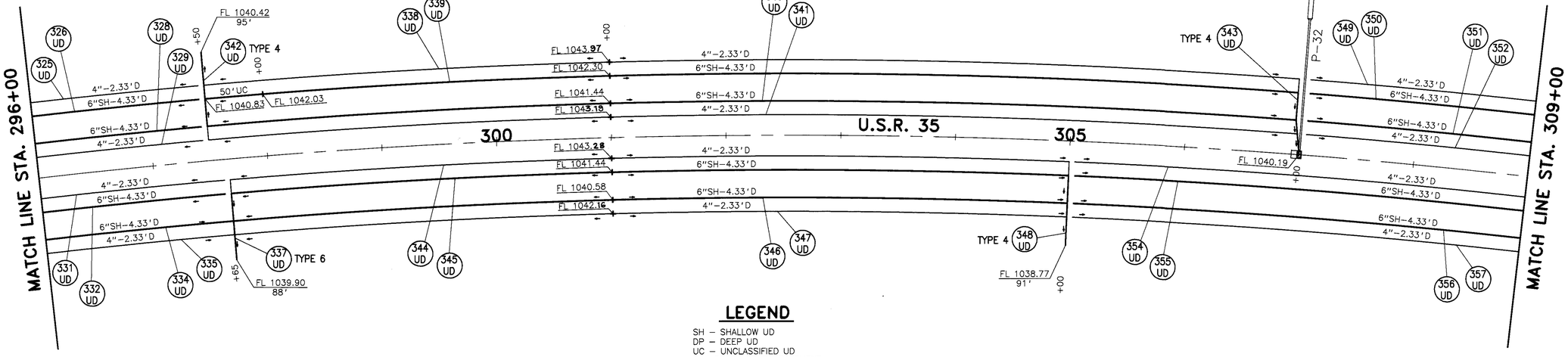
UNDERDRAINS – PROP. U.S.R. 35
STA. 256+00 TO STA. 296+00

CALCULATED | CHECKED
REVISED 10-99 BY G F R

HORIZONTAL SCALE IN FEET
0 25 50 100

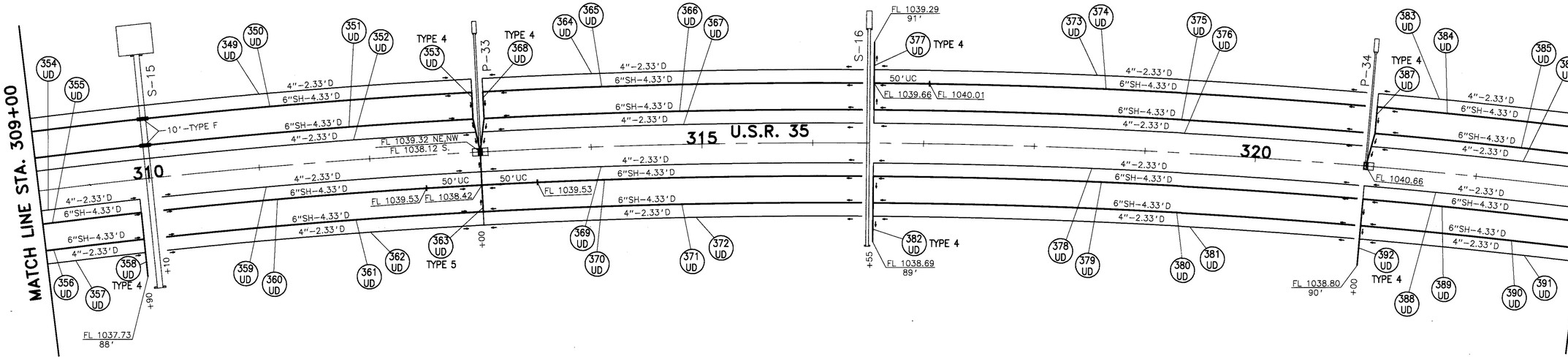
FAY-35-2.57

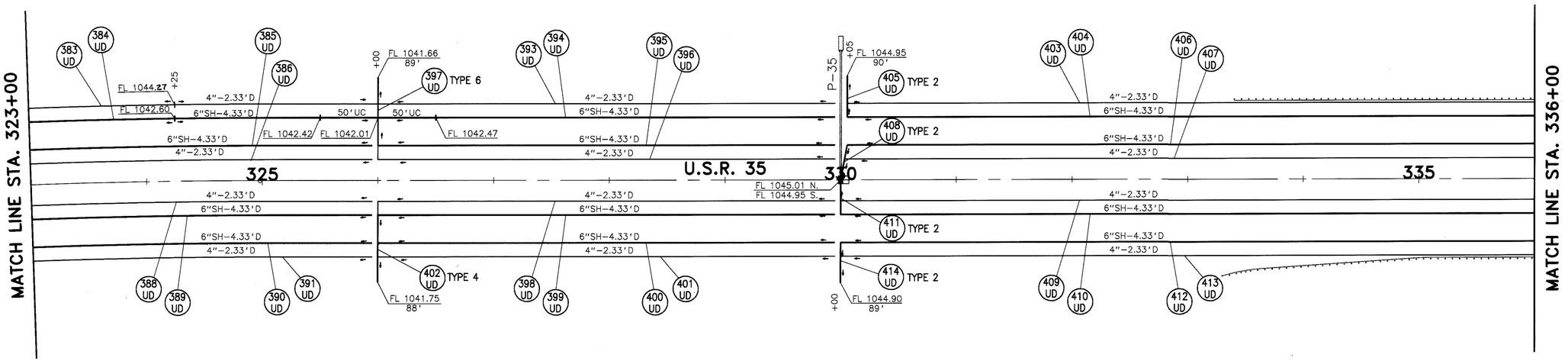
348
522



LEGEND

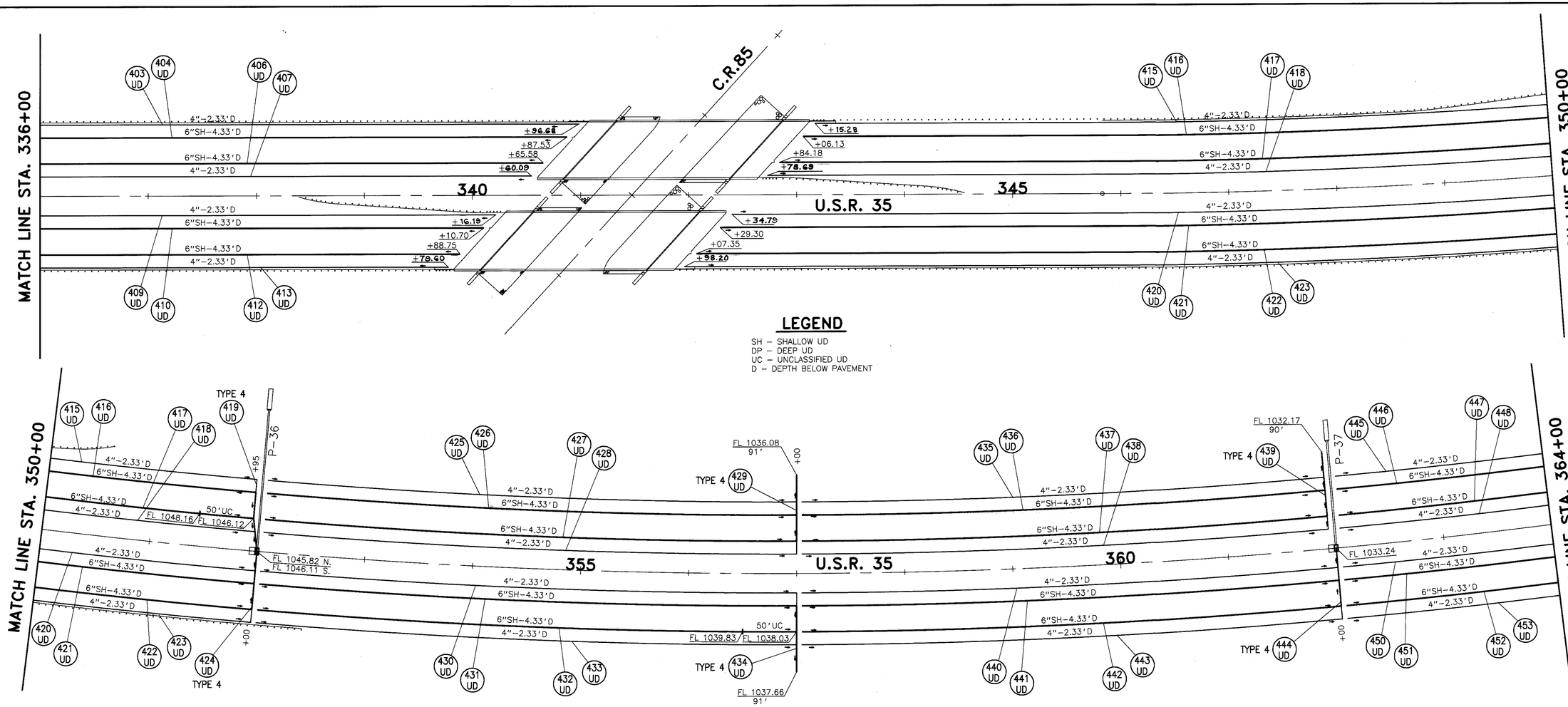
SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT





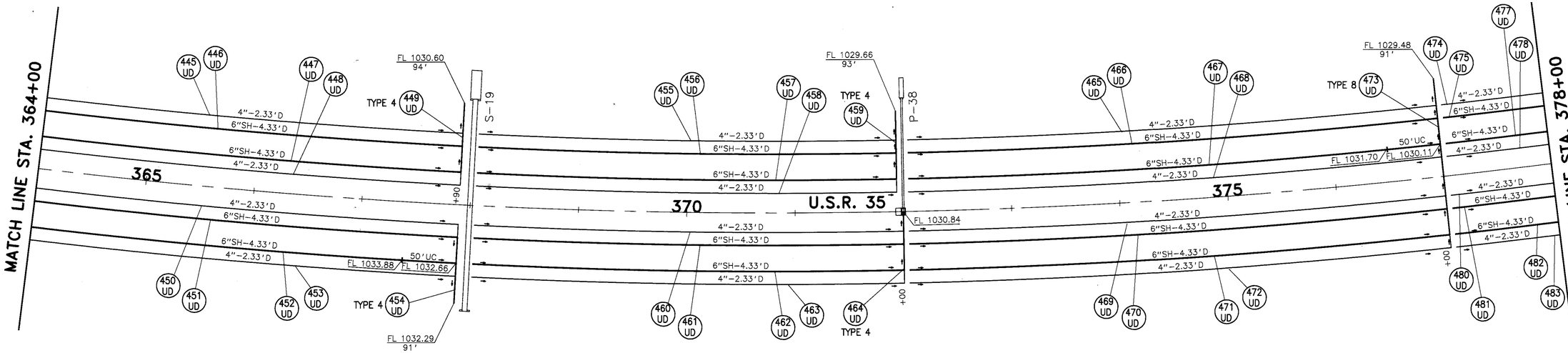
UNDERDRAINS – PROP. U.S.R. 35
STA. 296+00 TO STA. 336+00

FAY–35–2.57



LEGEND

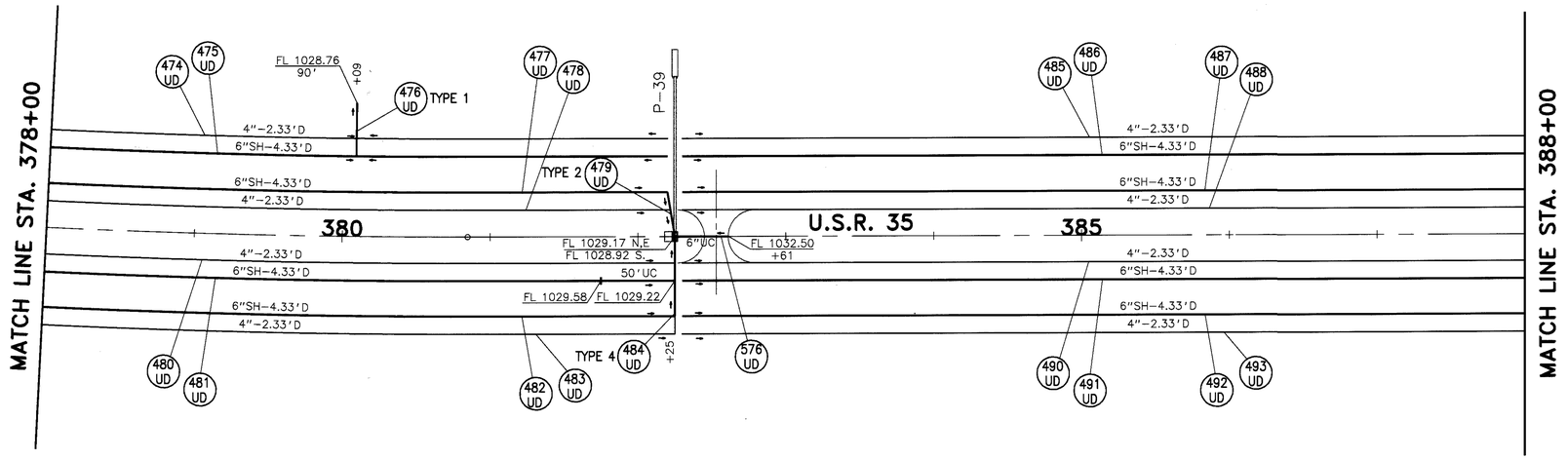
SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT



UNDERDRAINS – PROP. U.S.R. 35
STA. 336+00 TO STA. 378+00

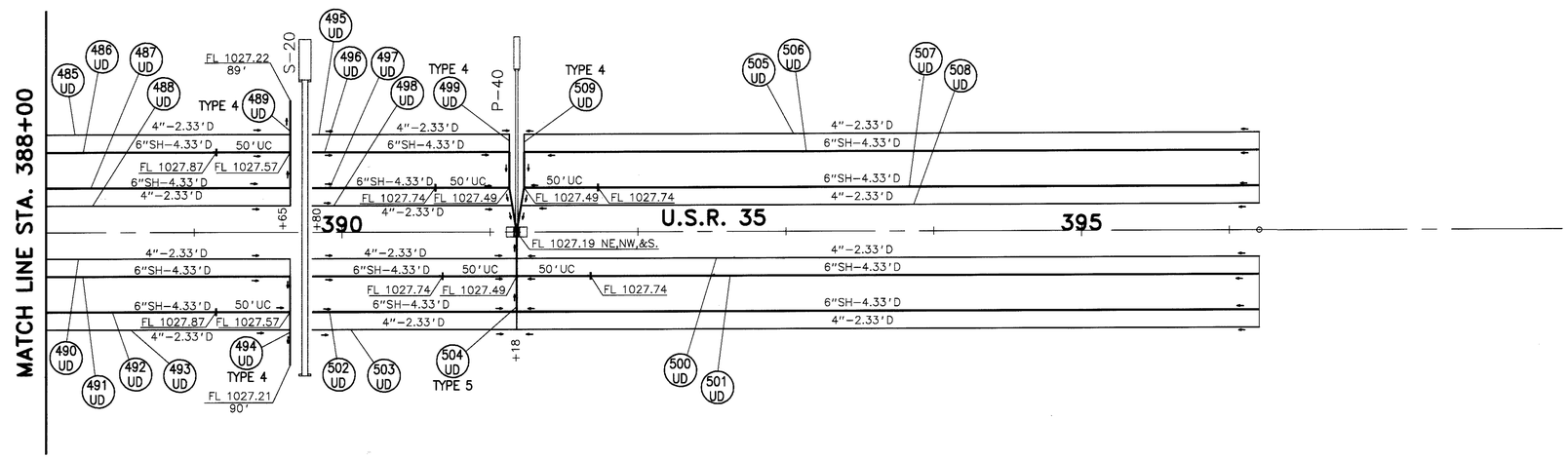
FAY–35–2.57

350
522



LEGEND

SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT



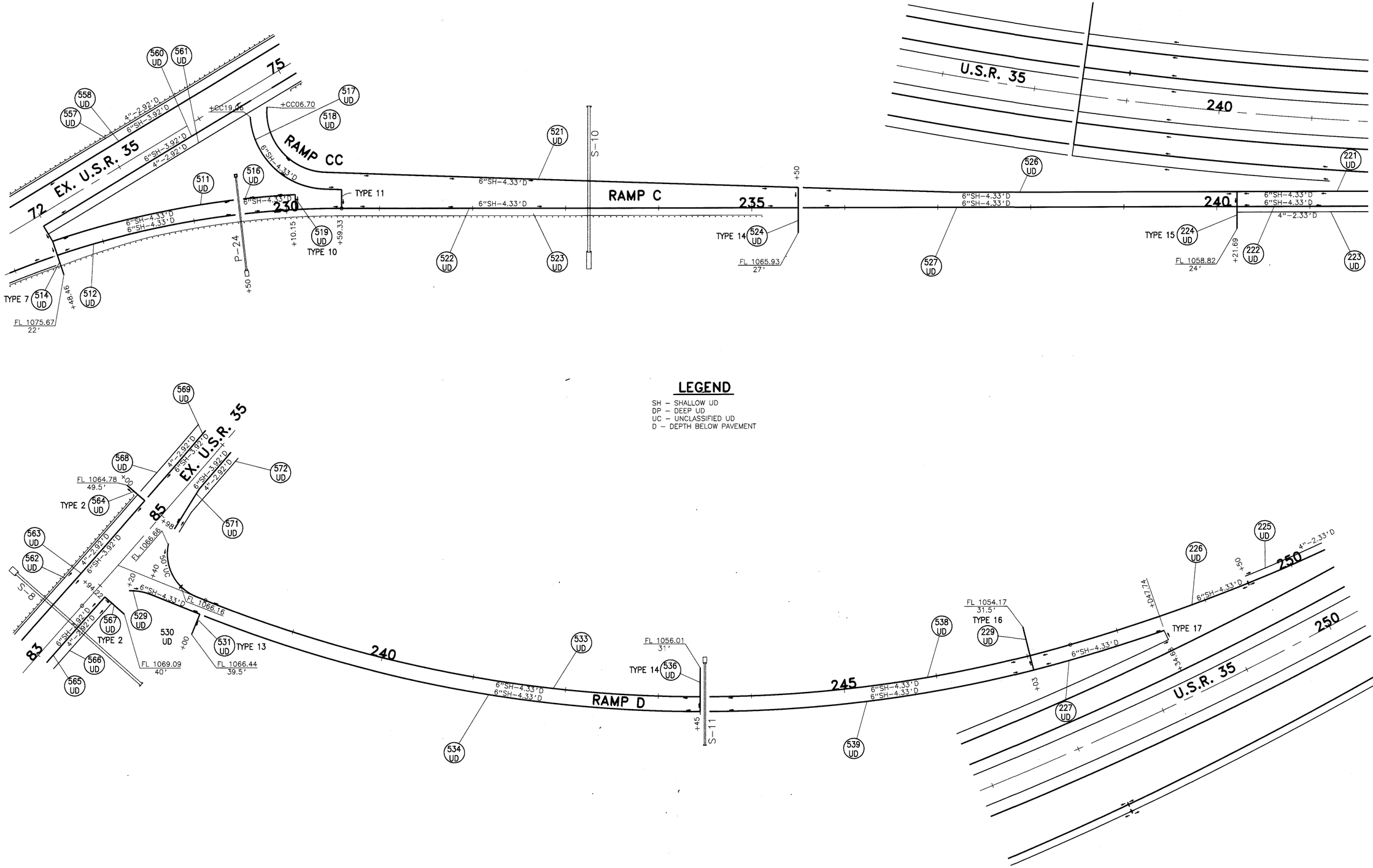
0 25 50 100 HORIZONTAL SCALE IN FEET

CALCULATED

CHECKED

UNDERDRAINS – PROP. U.S.R. 35
STA. 378+00 TO STA. 397+00

FAY-35-2.57



LEGEND

SH – SHALLOW UD
DP – DEEP UD
UC – UNCLASSIFIED UD
D – DEPTH BELOW PAVEMENT

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED
CHECKED

UNDERDRAINS – RAMP C & RAMP D
STA. C227+00 TO STA. C241+00 & STA. D237+00 TO STA. D250+00

FAY-35-2.57

352
522

CALCULATED

CHECKED

0 25 50 100 HORIZONTAL SCALE IN FEET

UNDERDRAINS - EXIST. U.S.R. 35
STA. 62+00 TO STA. 90+00

FAY-35-2.57

353
522

ITEM NO.	SHEET NOS. 355	356	357	358	373	ITEM NO.	ITEM EXT.	GRAND TOTAL	UNITS	DESCRIPTION
202		84				202	54000	84	EACH	RAISED PAVEMENT MARKER REMOVED
620		79				620	10300	79	EACH	DELINEATOR, TYPE C, POST MOUNTED
620		9				620	15300	9	EACH	DELINEATOR, TYPE D, POST MOUNTED
621		665				621	00100	665	EACH	RAISED PAVEMENT MARKER
625					2	625	32000	2	EACH	GROUND ROD
626		180				626	00100	180	EACH	BARRIER REFLECTOR, TYPE A
626		46				626	00200	46	EACH	BARRIER REFLECTOE, TYPE B
630			8	2		630	84500	10	EACH	GROUND MOUNTED BEAM SUPPORT FOUNDATION
630					2	630	84510	2	EACH	RIGID OVERHEAD SIGN SUPPORT FOUNDATION
630			90	65		630	02100	155	LIN. FT.	GROUND MOUNTED SUPPORT, NO. 2 POST
630			112	58		630	03100	170	LIN. FT.	GROUND MOUNTED SUPPORT, NO. 3 POST
630			42	16		630	04100	58	LIN. FT.	GROUND MOUNTED SUPPORT, NO. 4 POST
630				18		630	06100	18	LIN. FT.	GROUND MOUNTED SUPPORT, NO. 6 POST
630			33			630	06400	33	LIN. FT.	GROUND MOUNTED SUPPORT, S4X7.7 BEAM
630			39			630	06500	39	LIN. FT.	GROUND MOUNTED SUPPORT, W6X9 BEAM
630			94	49		630	07000	143	LIN. FT.	GROUND MOUNTED SUPPORT, W8X18 BEAM
630			79	43		630	07600	122	LIN. FT.	GROUND MOUNTED SUPPORT, W10X12 BEAM
630				14		630	08004	14	LIN. FT.	ONE WAY SUPPORT, NO. 3 POST
630			12	2		630	09000	14	EACH	BREAKAWAY BEAM CONNECTION
630					1	630	20500	1	EACH	OVERHEAD SIGN SUPPORT, TYPE TC-12.30, DESIGN 5
630					1	630	20800	1	EACH	OVERHEAD SIGN SUPPORT, TYPE TC-12.30, DESIGN 8
630			123	79		630	80100	202	SQ. FT.	SIGN, FLAT SHEET
630			63	76		630	80102	139	SQ. FT.	SIGN, FLAT SHEET, TYPE G
630			386	85	182	630	80204	653	SQ. FT.	SIGN, EXTRUSHEET, TYPE G
630			11	14		630	84900	25	EACH	REMOVAL OF GROUND MOUNTED SIGN AND DISPOSAL
630			13	8		630	86002	21	EACH	REMOVAL OF GROUND MOUNTED POST SUPPORT AND DISPOSAL
631					2	631	84000	2	EACH	SIGN SERVICE
631					2	631	84300	2	EACH	SIGN WIRED
631					2	631	85100	2	EACH	DISCONNECT SWITCH WITH ENCLOSURE, TYPE X
631					4	631	87200	4	EACH	BALLAST, TYPE CMRI-175-480, REMOTE
631					4	631	89200	4	EACH	MERCURY VAPOR LUMINAIR, TYPE TC-31.21 WITH 175 WATT LAMP
642	2.46	1.95				642	00100	4.41	MILE	EDGE LINE, TYPE I
642	0.65					642	00200	0.65	MILE	LANE LINE, TYPE I
642	1.09	0.99				642	00300	2.08	MILE	CENTER LINE, TYPE I
642	122					642	00400	122	LIN. FT.	CHANNELIZING LINE, TYPE I
642	168	31				642	00500	199	LIN. FT.	STOP LINE, TYPE I
642	571					642	00700	571	LIN. FT.	TRANSVERSE LINE, TYPE I
828	20.89					828	10000	20.89	MILE	EDGE LINE
828	9.97					828	10100	9.97	MILE	LANE LINE
828	832					828	10300	832	LIN. FT.	CHANNELIZING LINE

CALCULATED J.L.G. CHECKED W.G.Y.

REVISED 12/99 BY FYC

TRAFFIC CONTROL GENERAL SUMMARY

FAY-35-2.57

1 OF 30

354 / 522

01-FEB-2000 2:36PM fcheek

I:\pr\project\fay35\fay-35-2.57\fay354gg.dgn

I:\pr\project\fay35\fay-35-2.57\ fay354gg.dgn

Revised 8/08/00 by E.R.P. 10/10/00 by T.H.S.

PAVEMENT MARKING SUB-SUMMARY

CALCULATED	J.L.G.
CHECKED	W.G.Y.
REVISED	12/99
BY	FYC

FAY-35-2.57

2 OF 30

355/522

SHEET NO.	LOCATION	STATION FROM	STATION TO	SIDE	642 EDGE LINE, YELLOW LIN. FT.	642 EDGE LINE, WHITE LIN. FT.	642 LANE LINE LIN. FT.	642 CENTER LINE, DOUBLE SOLID LIN. FT.	642 CHANNELIZING LINE LIN. FT.	642 STOP LINE LIN. FT.	642 TRANSVERSE LINE, YELLOW LIN. FT.	828 EDGE LINE, YELLOW LIN. FT.	828 EDGE LINE, WHITE LIN. FT.	828 LANE LINE LIN. FT.	828 CHANNELIZING LINE LIN. FT.
359,360,361	PROP. U.S.R. 35	135+72.80	248+38.05	LT.									11300		
361,362,363,364	PROP. U.S.R. 35	253+00.00	396+20.24	LT.									14362		
359-364	PROP. U.S.R. 35	135+72.80	396+20.24	L.&R.								52114			
359-364	PROP. U.S.R. 35	135+72.80	396+20.24	L.&R.										52114	
361	PROP. U.S.R. 35	251+21.53	253+29.19	LT.										212	
361,369	PROP. U.S.R. 35	247+44.60	250+32.44	RT.										317	
359,360,361	PROP. U.S.R. 35	135+72.80	248+30.14	RT.									11300		
361,362,363,364	PROP. U.S.R. 35	242+00.00	396+20.24	RT.									15418		
369	PROP. U.S.R. 35	245+99.68	247+44.60	RT.											147
369	PROP. U.S.R. 35	248+38.05	251+22.28	LT.											280
366,367,369	RAMP "C"	225+40.19	242+00.00	RT.									1690		
366	RAMP "C"	226+26.90	227+48.42	LT.							108				124
366,367	RAMP "C"	227+48.42	231+79.27	LT.								423			
367	RAMP "C"	237+19.33	245+98.61	LT.								898			
367	RAMP "CC"	229+01.08	230+59.33	℄		159									
367	RAMP "CC"	228+79.39	237+19.33	LT.	2693										
367,369	EX-USR35-RAMP "D"	83+94.22	248+47.74	RT.								1162			
367,369	EX-USR35-RAMP "D"	85+47.99	253+00.00	LT.									1637		
367	RAMP "D"	237+21.81		L.&R.						69					
369	RAMP "D"	248+47.74	251+22.28	RT.							222				281
366	EX. U.S.R. 35	62+25.00	65+19.44	LT.		317									
366,367,368	EX. U.S.R. 35	66+43.89	90+00.00	LT.		2376									
366	EX. U.S.R. 35	62+25.00	65+25.09	RT.		317									
366	EX. U.S.R. 35	66+50.69	69+87.48	RT.		317									
366,367	EX. U.S.R. 35	71+97.56	74+57.81	RT.		264									
366	EX. U.S.R. 35	70+75.52	71+97.56	RT.					122						
367	EX. U.S.R. 35	74+84.06	83+94.22	RT.		898									
367,368	EX. U.S.R. 35	85+47.99	90+00.00	RT.		476									
366	EX. U.S.R. 35	62+25.00	65+33.21	Q&LT.				634			212				
366	EX. U.S.R. 35	66+32.42	68+85.00	℄				528			29				
366,367,368	EX. U.S.R. 35	68+85.00	90+00.00	℄				2112							
				℄											
366,367,368	EX. U.S.R. 35	62+25.00	87+95.79	LT			2588								
366	EX. U.S.R. 35	62+25.00	70+75.52	RT.			845								
366	SERVICE RD. NO. 2	8+00.00	65+19.44	RT.		212									
366	SERVICE RD. NO. 2	8+00.00	66+43.89	LT.		212									
366	SERVICE RD. NO. 2	8+00.00	9+58.82	℄				159							
366	SERVICE RD. NO. 2	9+58.82		RT.						29					
366	SERVICE RD. NO. 1	65+25.09	13+00.00	RT.		317									
366	SERVICE RD. NO. 1	66+50.69	13+00.00	LT.		317									
366	SERVICE RD. NO. 1	10+34.52	13+00.00	℄				264							
366	SERVICE RD. NO. 1	10+34.52		LT.						39					
368	SERVICE RD. NO. 3	15+50.00	93+62.00	LT.		1498									
368	SERVICE RD. NO. 3	15+50.00	92+38.00	RT.		1445									
368	SERVICE RD. NO. 3	15+50.00	29+78.00	℄				1445							
368	SERVICE RD. NO. 3	29+78.00		RT.						31					
	HARMONY ROAD	24+50.00	30+44.00	L&R	594	594		594							
SUB-TOTALS					3287 L.F.	9719 L.F.	3433 L.F.	5736 L.F.				54597 L.F.	55707 L.F.	52643 L.F.	
TOTALS CARRIED TO GENERAL SUMMARY					13006 LIN. FT. 2.46 MI.		0.65 MI.	1.09 MI.	122	168	571	110,304 LIN. FT. 20.89 MI.		9.97 MI.	832

fcheek
01-FEB-2000 2:42PM
i:\pr\project\fay35\fay-35-2.57\fay355gb.dgn
11\pr\project\fay35\fay-35-2.57\ fay355gb.dgn

Revised 8/08/00 by E.R.P.

SHEET NO.	LOCATION	STATION FROM	STATION TO	SIDE	202 RAISED PAVEMENT MARKER REMOVED	626 BARRIER REFLECTOR, TYPE A	626 BARRIER REFLECTOR, TYPE B	642 EDGE LINE, WHITE	642 LANE LINE	642 CENTER LINE, BROKEN	642 CENTER LINE, DOUBLE SOLID	642 CENTER LINE, DOUBLE BROKEN AND SOLID	642 CHANNELIZING LINE		642 STOP LINE	620 DELINEATOR TYPE C POST MOUNTED	620 DELINEATOR TYPE D POST MOUNTED	621 RAISED PAVEMENT MARKER 1-WAY WHITE	621 RAISED PAVEMENT MARKER 1-WAY YELLOW	621 RAISED PAVEMENT MARKER 2-WAY YELLOW/ YELLOW	621 RAISED PAVEMENT MARKER 2-WAY WHITE/ RED	621 RAISED PAVEMENT MARKER 2-WAY YELLOW/ RED
					EACH	EACH	EACH	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.		LIN. FT.	EACH	EACH	EACH	EACH	EACH	EACH	EACH
365	C.R. 16	20+00.00	20+75.00	℄						106												
365	C.R. 16	20+75.00	28+75.00	℄								792										
365	C.R. 16	28+75.00	30+00.00	℄						106												
365	C.R. 16	30+00.00	33+35.00	℄								317										
365	C.R. 16	33+90.00	39+00.00	℄								528										
365	C.R. 16	20+00.00	39+00.00	LT.				1901														
365	C.R. 16	20+00.00	33+02.74	RT.				1320														
365	C.R. 16	34+22.71	39+00.00	RT.				476														
365	T.R. 94	33+02.74	4+00.00	RT.				423														
365	T.R. 94	34+22.71	4+00.00	LT.				423														
365	T.R. 94	0+21.00		LT.											31							
365	T.R. 94	0+21.00	2+00.00	℄								212										
365	T.R. 94	2+00.00	4+00.00	℄						212												
370	S.R. 729	152+75.00	181+44.00	L.&R.				5756														
370	S.R. 729	152+75.00	156+25.00	℄						370												
370	S.R. 729	156+25.00	166+25.00	℄								1004										
370	S.R. 729	166+25.00	167+00.00	℄							106											
370	S.R. 729	167+00.00	175+75.00	℄								898										
370	S.R. 729	175+75.00	177+00.00	℄						106												
370	S.R. 729	177+00.00	181+44.00	℄								476										
370	S.R. 729	171+00.00	181+44.00	RT.	26																	
370	S.R. 729	179+45.97	181+44.00	LT.	11																	
370	S.R. 729	152+85.06	181+44.00	℄	47																	
374	PROP. U.S.R. 35	135+72.80	185+00.00	L.&R.												8		82				
375	PROP. U.S.R. 35	185+00.00	240+00.00	L.&R.												22		92				
376	PROP. U.S.R. 35	240+00.00	265+00.00	L.&R.												19	4	40			21	19
377	PROP. U.S.R. 35	265+00.00	313+00.00	L.&R.												14		80				
378	PROP. U.S.R. 35	313+00.00	365+00.00	L.&R.												18		88				
379	PROP. U.S.R. 35	365+00.00	396+20.24	L.&R.												8		52				
380	EX. U.S.R. 35	60+00.00	73+00.00	–												3	1		26	6	28	2
381	EX. U.S.R. 35	73+00.00	87+00.00	–												6	4			17	18	38
382	EX. U.S.R. 35	87+00.00	95+00.00	–																4		
383	S.R. 729	152+00.00	182+00.00	–														16		36		
359	PROP. U.S.R. 35	137+05.91	138+90.25	RT.		1	2															
359	PROP. U.S.R. 35	137+35.35	139+19.69	RT.		1	2															
359	PROP. U.S.R. 35	136+80.31	138+64.65	LT.		1	2															
359	PROP. U.S.R. 35	137+09.75	138+94.09	LT.		1	2															
359	PROP. U.S.R. 35	162+38.00	162+88.00	RT.		3																
359	PROP. U.S.R. 35	162+30.00	162+80.00	LT.		3																
359,360	PROP. U.S.R. 35	182+60.91	209+80.82	LT.		25	3															
360	PROP. U.S.R. 35	193+13.37	196+35.50	RT.		1	3															
359,360	PROP. U.S.R. 35	184+74.81	210+31.85	RT.		23	3															
360	PROP. U.S.R. 35	193+29.98	196+53.26	LT.		1	3															
360	PROP. U.S.R. 35	231+26.06	232+75.79	RT.		3																
360	PROP. U.S.R. 35	231+14.40	232+77.19	LT.		3																
362	PROP. U.S.R. 35	273+13.06	273+63.06	RT.		3																
362	PROP. U.S.R. 35	273+10.56	273+60.56	LT.		3																
363	PROP. U.S.R. 35	333+46.62	348+69.25	LT.		14	2															
363	PROP. U.S.R. 35	335+16.63	352.40.46	RT.		16	2															
363	PROP. U.S.R. 35	340+13.64	342+34.87	RT.		1	2															
363	PROP. U.S.R. 35	340+60.01	342+81.24	LT.		1	2															
365	C.R. 16	26+37.67	32+80.40	LT.		5	3															
365	C.R. 16	26+19.60	32+62.33	RT.		5	3															
366,367	EX. U.S.R. 35	68+85.47	84+96.01	LT.		14	3															
366,367	EX. U.S.R. 35	74+99.94	79+33.90	RT.		3	3															
366,367	RAMP C	68+02.98 EX.35	35+11.05 RAMP C	LT.		18																
370	S.R. 729	156+92.11	162+04.61	LT.		6																
370	S.R. 729	162+66.63	174+04.39	LT.		10	3															
370	S.R. 729	156+93.91	174+06.77	RT.		15	3															
SUB–TOTALS								10299 L.F.		900 L.F.	106 L.F.	4227 L.F.						450	26	63	67	59
TOTALS CARRIED TO GENERAL SUMMARY					84	180	46	1.95 MI.		5233 LIN. FT. 0.99 MI.					31	79	9	665				

CALCULATED J.L.G.
CHECKED W.G.Y.

PAVEMENT MARKING SUB-SUMMARY

FAY-35-2.57

3 OF 30

356
522

Q:\3590\TRA_CONL\3590pms2

SHEET NO.	REFERENCE NO.	STATION	SIDE	CODE	SIZE (INCHES)	POST LOCATION OFFSET FROM EDGE OF PAVEMENT		630													
								SIGN, FLAT SHEET	SIGN, FLAT SHEET, TYPE G	GROUND MOUNTED SUPPORTS			GROUND MOUNTED SUPPOER, S4X7.7 BEAM	GROUND MOUNTED SUPPOER, W6X9 BEAM	GROUND MOUNTED SUPPORT, W8x18 BEAM	GROUND MOUNTED SUPPORT, W10x12 BEAM	BREAKAWAY BEAM CONNECTION	REMOVAL OF GROUND MOUNTED SIGN & DISPOSAL	REMOVAL OF GROUND MOUNTED POST SUPPORT & DISPOSAL	SIGN, EXTRUSHEET, TYPE G	GROUND MOUNTED BEAM SUPPORT FOUNDATION
						POST 1	POST 2			POSTS 2	3	4									
						FEET	FEET	SQ. FT.	SQ. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	EACH	EACH	EACH	SQ. FT.	EACH
361	1	STA. 240+00	RT.	W-49R-48	48x48	19.6	24.1	16				14/14									
	2	STA. 248+08	LT.	GF	72x60	16.12	19.48						16/16.4				2			30	
	3	STA. 256+38	LT.	GE	144x96	31.75	40.25								49		2			96	2
361	4	STA. 263+00	RT.	M-1-30	30x30	19.5		6.25				13.5									
				M-39-30	30x15			3.13													
362	5	STA. 273+00	RT.	R-9-84	84x72	19.54	23.46	42						19.15/19.55			2				
	6	STA. 273+60	LT.	GE	156x96	32.25	40.75								45		2			104	2
	7	STA. 280+05	℄	R-123-36	36x36	30			9		13.5										
				R-19	24x30			5													
	8	STA. 280+51	℄	R-123-36	36x36	30			9		13.9										
				R-19	24x30			5													
	9	STA. 295+00	RT.	GJ	180x48	35.25	43.75									17.6/16.8	2			60	
362	10	STA. 301+00	LT.	GE	144x96	31.75	40.25									44	2			96	2
364	11	STA. 382+30	℄	R-123-36	36x36	30			9		13.5										
				R-19	24x30			5													
364	12	STA. 382+76	℄	R-123-36	36x36	30			9		13.9										
				R-19	24x30			5													
365	14	STA. 33+64 C.R. 16	LT.															3	3		
	15	STA. 33+70 C.R. 16 & STA. 33+74 C.R. 16	LT.	W-30-48	48x24	12	12		8	12/12											
	16	STA. 0+33 T.R. 94	LT.															1	1		
365	17	STA. 0+40 T.R. 94	LT.	R-1-30	30x30	13.25			6.25	13.25											
366	18	STA. 60+00 EX.U.S.R. 35	RT.	M-1-24-2	24x24	13		4		13.3											
				M-17	21x15			2.19													
	19	STA. 63+77 EX.U.S.R. 35	RT.															1	1		
	20	STA. 63+78 EX.U.S.R. 35	RT.	R-10-36	36x48	16.5	18.5	12			13.9/14.4										
	21	STA. 63+78 EX.U.S.R. 35	LT.															1	2		
	22	STA. 63+83 EX.U.S.R. 35	LT.	R-10-36	36x48	16.5	18.5	12			13.9/14.4										
	23	SEE SHEET 373																			
	24	STA. 65+48 EX.U.S.R. 35	LT.	R-1-30	30x30	13.25			6.25	13											
	25	STA. 66+22 EX.U.S.R. 35	RT.	R-1-30	30x30	13.25			6.25	13											
	26	STA. 67+67 EX.U.S.R. 35	LT.															1	1		
	27	STA. 68+95 EX.U.S.R. 35	LT.															1	2		
	28	STA. 69+28 EX.U.S.R. 35	LT.	R-11A-24	24x30	17		5		13											
	29	SEE SHEET 373																			
	30	STA. 71+00 EX.U.S.R. 35	RT.															1	1		
	31	STA. 71+50 EX.U.S.R. 35	LT.															1	1		
366	32	STA. 71+78 EX.U.S.R. 35	RT.															1	1		
TOTALS								122.57	62.75	89.55	111.4	41.5	32.4	38.7		78.4					
TOTALS CARRIED TO GENERAL SUMMARY								123	63	90	112	42	33	39	94	79	12	11	13	386	8

CALCULATED J.L.G.
CHECKED W.G.Y.

SIGNING SUB-SUMMARY

FAY-35-2.57

Q:\3590\TRA_CONL\3590SM1

SHEET NO.	REFERENCE NO.	STATION	SIDE	CODE	SIZE (INCHES)	630														
						POST LOCATION OFFSET FROM EDGE OF PAVEMENT		SIGN, FLAT SHEET	SIGN, FLAT SHEET, TYPE G	ONE WAY SUPPORT, NO.3 POST	GROUND MOUNTED SUPPORTS POSTS				GROUND MOUNTED SUPPORT, W8x18 BEAM	GROUND MOUNTED SUPPORT, W10X12 BEAM	BREAKAWAY BEAM CONNECTION	REMOVAL OF GROUND MOUNTED SIGN & DISPOSAL	REMOVAL OF GROUND MOUNTED POST SUPPORT & DISPOSAL	SIGN, EXTRUSHEET, TYPE G	GROUND MOUNTED BEAM SUPPORT FOUNDATION
						POST 1	POST 2				2	3	4								
						FEET	FEET	SQ. FT.	SQ. FT.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.		LIN. FT.	LIN. FT.	EACH	EACH	EACH	SQ. FT.	EACH
367	33	STA. 73+93 EX.U.S.R. 35	℄															1	1		
↓	34	STA. 230+00 RAMP C	RT.	W-49L-36	36x36	14.12		9					16								
	35	STA. 75+00 EX.U.S.R. 35	LT.	GB	156x78	18.25	26.75								22.2/26.4					84.5	2
	36	STA. 79+50 EX.U.S.R. 35	LT.	M-45-42	42x60	16.25	18.75		17.5				17/18								
	37	STA. 237+25 RAMP D	RT.	R-41B-36	36x36	14.25			9	14											
				R-43L-36	36x12				3												
				R-43R-36	36x12				3												
	38	STA. 237+60 RAMP D	LT.	R-41B-36	36x36	17.5	19.5		9			14/14									
				R-1-48	48x48				16												
↓	39	STA. 239+50 RAMP D	RT.	R-41A-36	36x24	13.5			6		13										
367	40	STA. 239+50 RAMP D	LT.	R-41A-36	36x24	13.5			6		12										
368	41	STA. 87+50 EX.U.S.R. 35	LT.	M-1-24-2	24x24	17		4			14										
				M-17	21x15			2.19													
368	42	STA. 29+78 SERVICE RD. NO.3	RT.	R-1-30	30x30	13.25			6.25		13										
369	43	STA. 244+00 RAMP D	LT.	IM-8-30	30x15	13.76		3.13								21/22	2				
				M-5-30-2	30x30			6.25													
				IM-24-24	24x18			3													
				IM-8-30	30x15		18.24	3.13													
				M-2-30-3	37.5x30			7.81													
				IM-24-24	24x18			3													
				D-4D	96x24			16													
370	44	STA. 173+45 S.R. 729	RT.															1	1		
↓	45	STA. 173+75 S.R. 729	RT.	W-45A-36	36x36	14.12		9				16									
	46	STA. 176+19 S.R. 729	RT.															2	1		
	47	STA. 177+73 S.R. 729	RT.															2	1		
	48	STA. 177+73 S.R. 729	RT.	M-2-24-3	24x24	13		4				14									
				M-19-21	21x15			2.19													
	49	STA. 180+30 S.R. 729	LT.															2	1		
	50	STA. 180+30 S.R. 729	LT.	M-2-24-3	24x24	19		4			13										
				M-38-24	24x12			2													
	51	STA. 108+04 EX.U.S.R. 35	LT.															2	1		
	52	STA. 108+79 EX.U.S.R. 35	LT.															2	1		
370	53	STA. 110+63 EX.U.S.R. 35	RT.															2	1		
TOTALS								78.7	75.75						48.6					84.5	
TOTALS CARRIED TO GENERAL SUMMARY								79	76	14	65	58	51		49	43	2	14	8	85	2

CALCULATED J.L.G.
CHECKED W.G.Y.

SIGNING SUB-SUMMARY

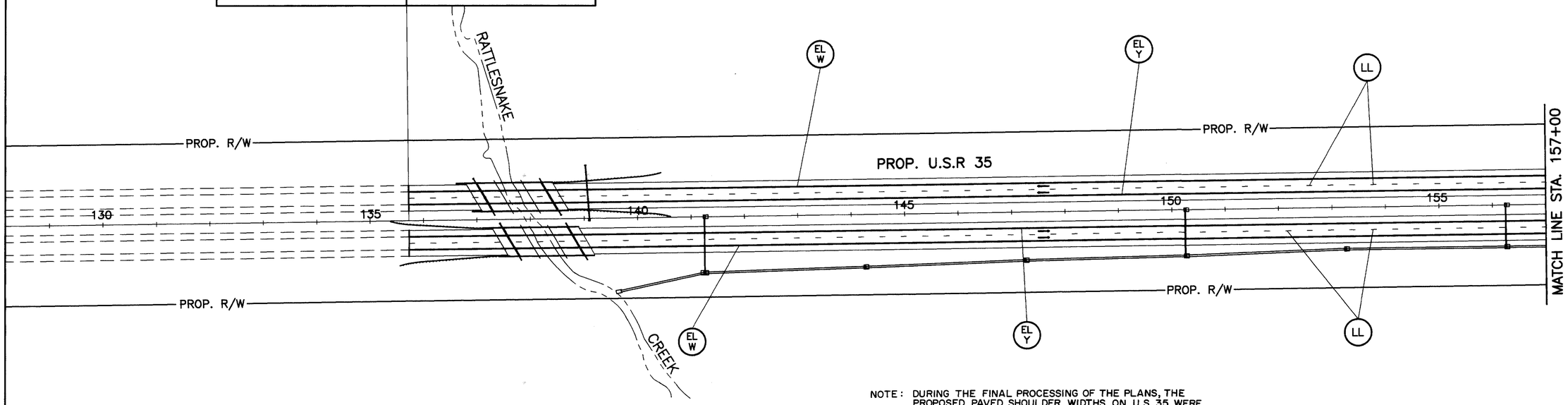
FAY-35-2.57

358
522

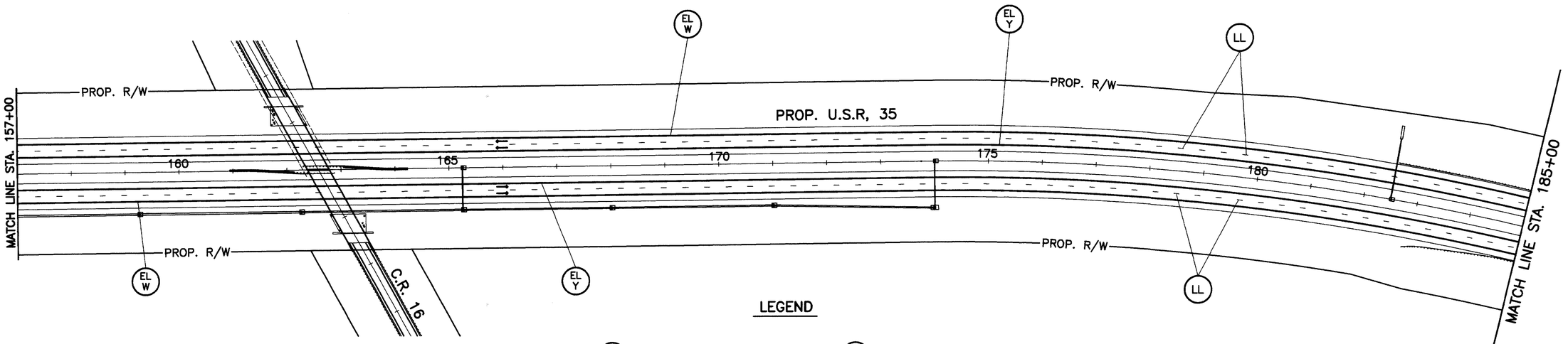
Q:\3590\TRA_CONL\3590SM2

END PROJECT
GRE\FAY-35-20/0.00
STA. 871+46.62 BK.

BEGIN PROJECT
STA. 135+72.80 AHD.
S.L.M. 2.57



NOTE: DURING THE FINAL PROCESSING OF THE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE SIGNING DETAIL SHEETS WERE REVISED. THE TRAFFIC CONTROL PLAN SHEETS AND THE SIGNING & STRIPING SHEETS WERE NOT REVISED.



LEGEND

SIGNS

- EXISTING
- EXISTING TO BE REMOVED
- PROPOSED

LL	LANE LINE
EL W	EDGE LINE, WHITE
EL Y	EDGE LINE, YELLOW
CH	CHANNELIZING LINE
WT	TRANSVERSE LINE, WHITE
YT	TRANSVERSE LINE, YELLOW
SL	STOP LINE
CC	CENTER LINE, BROKEN & SOLID, DOUBLE
C	CENTER LINE, BROKEN, SINGLE
CD	CENTER LINE, SOLID, DOUBLE

NOTES

FOR QUANTITIES, SEE SHEET NO. 355

FOR C.R. 16, SEE SHEET NO. 365

HORIZONTAL SCALE IN FEET
0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

REVISED 10-99 BY F Y C

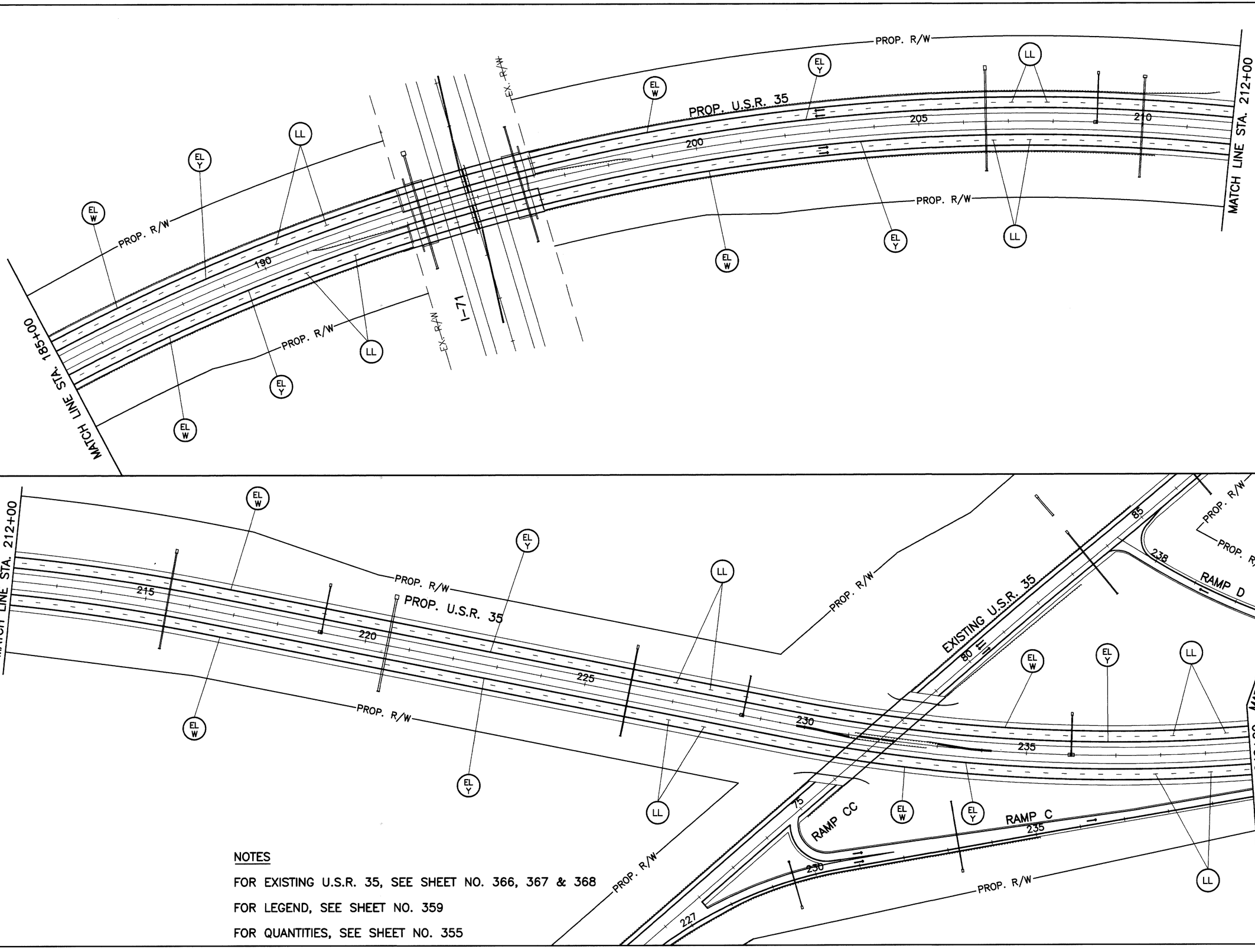
TRAFFIC CONTROL
STA. 130+00 TO STA. 185+00

FAY-35-2.57

6 OF 30

359
522

Q:\3590\TRA_CONL\3590TC1



NOTES

FOR EXISTING U.S.R. 35, SEE SHEET NO. 366, 367 & 368

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355

CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 50 100 200

TRAFFIC CONTROL
185+00 TO STA. 240+00

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC2

NOTES

1. FOR RAMPS C & D STRIPING AT THE NOSES, SEE SHEET 369 FOR LAYOUT DETAILS.
2. FOR LEGEND, SEE SHEET NO. 359
3. FOR QUANTITIES, SEE SHEET NO. 355 & 357

CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 50 100 200

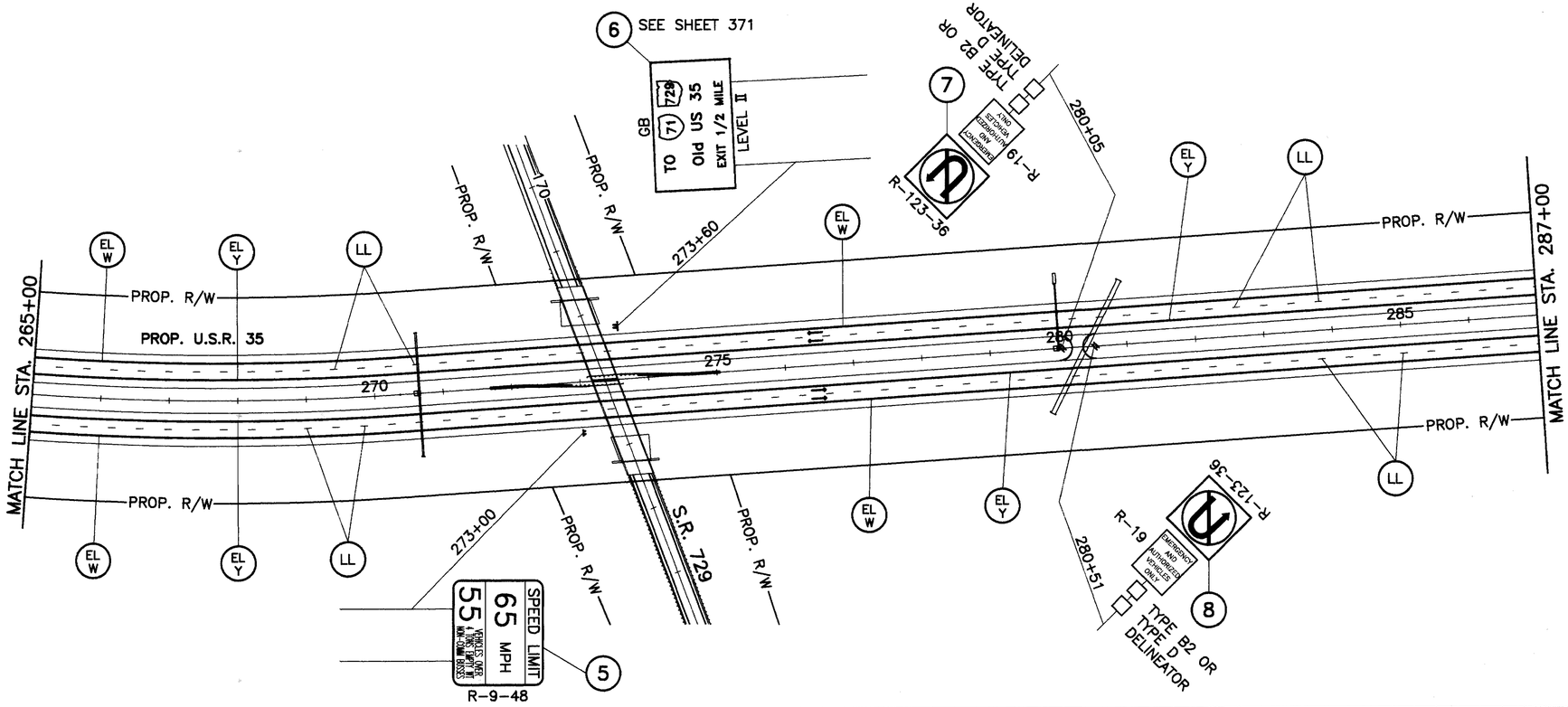
TRAFFIC CONTROL
STA. 240+00 TO STA. 265+00

FAY-35-2.57

8 OF 30

361
522

Q:\3590\TRA_CONL\3590T32



NOTES

FOR LEGEND, SEE SHEET NO. 359

FOR S.R. 729, SEE SHEET NO. 370

FOR QUANTITIES, SEE SHEET NO. 355 & 357

CALCULATED M.T.
CHECKED W.G.Y.

SCALE IN FEET
0 50 100 200

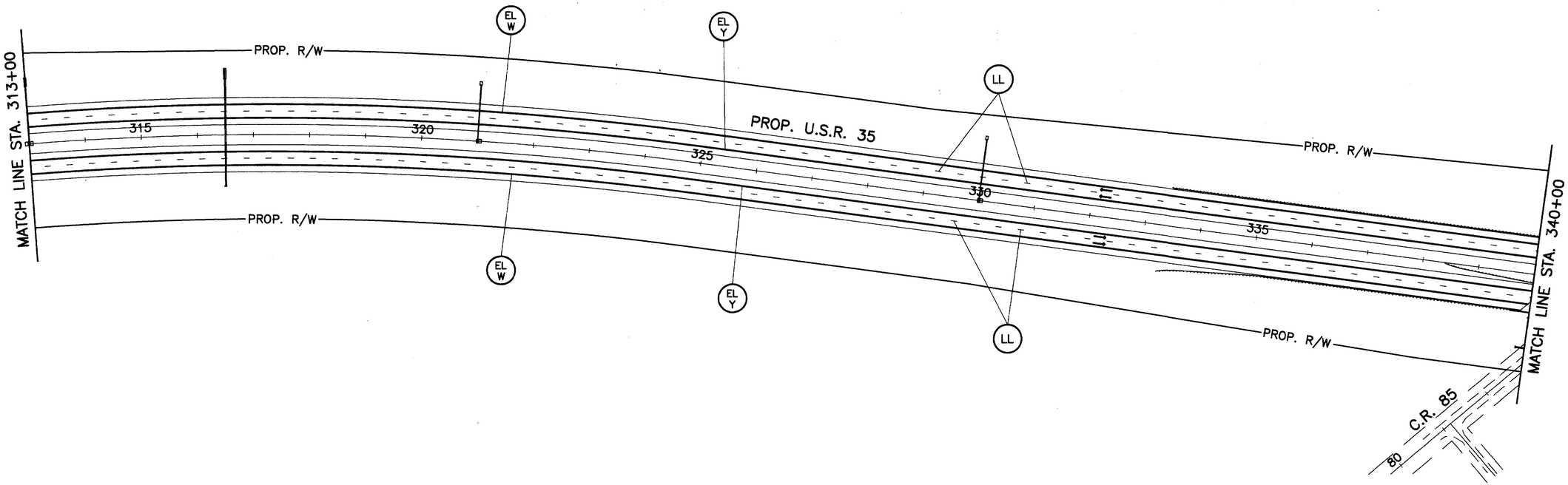
TRAFFIC CONTROL
STA. 265+00 TO STA. 313+00

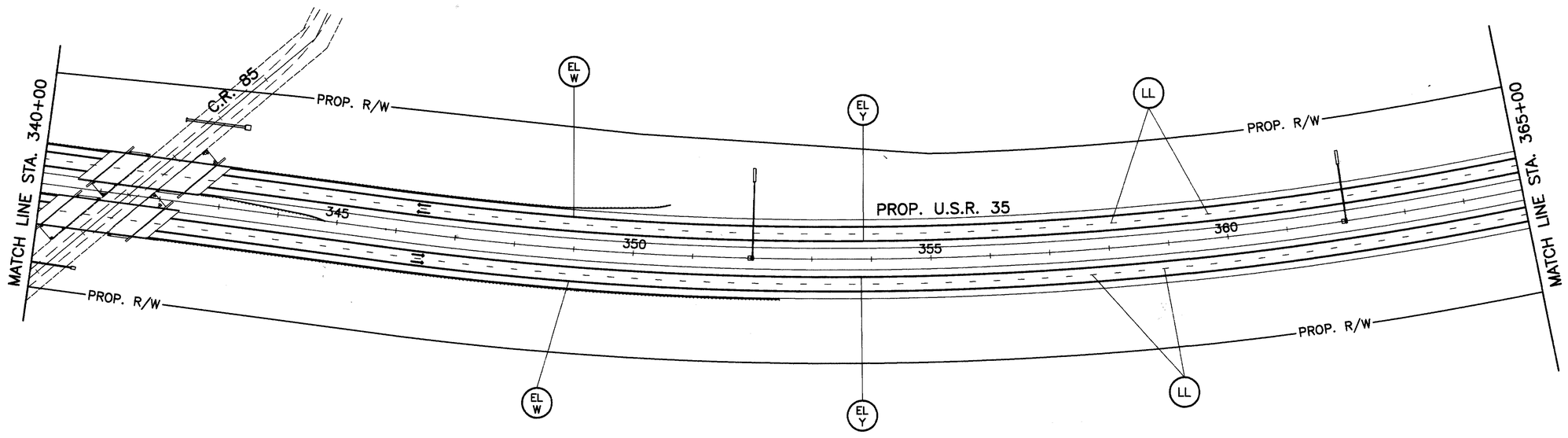
FAY-35-2.57

9 OF 30

362
522

Q:\3590\TRA_CONL\3590TC4





NOTES

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355

HORIZONTAL SCALE IN FEET
0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

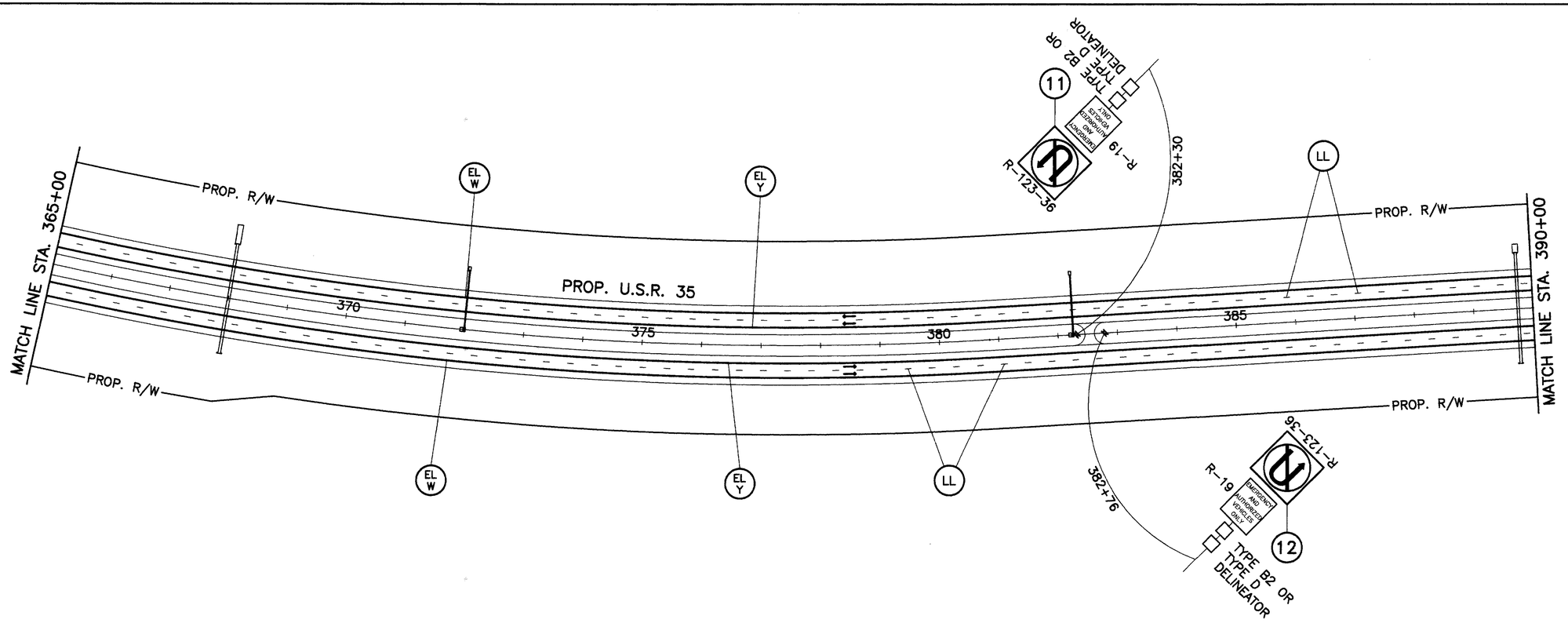
TRAFFIC CONTROL
STA. 313+00 TO STA. 365+00

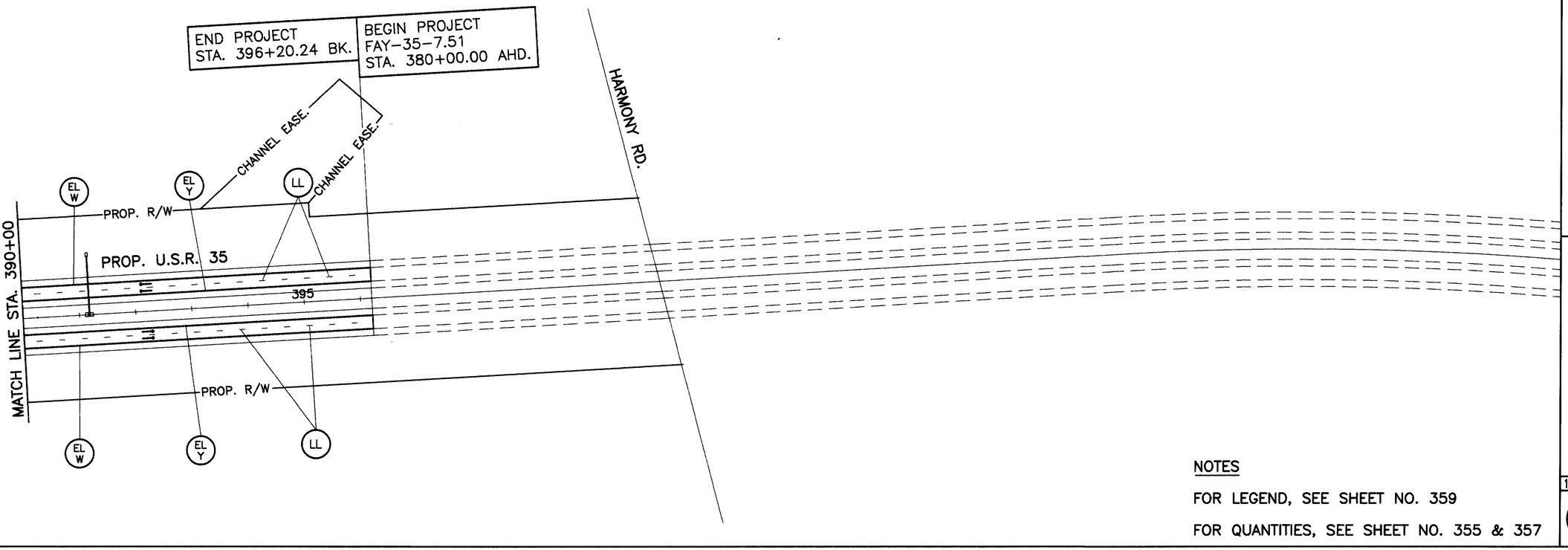
FAY-35-2.57

10 OF 30

363
522

Q:\3590\TRA_CONL\3590TC5





NOTES

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355 & 357

CALCULATED M.T. CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET 0 50 100 200

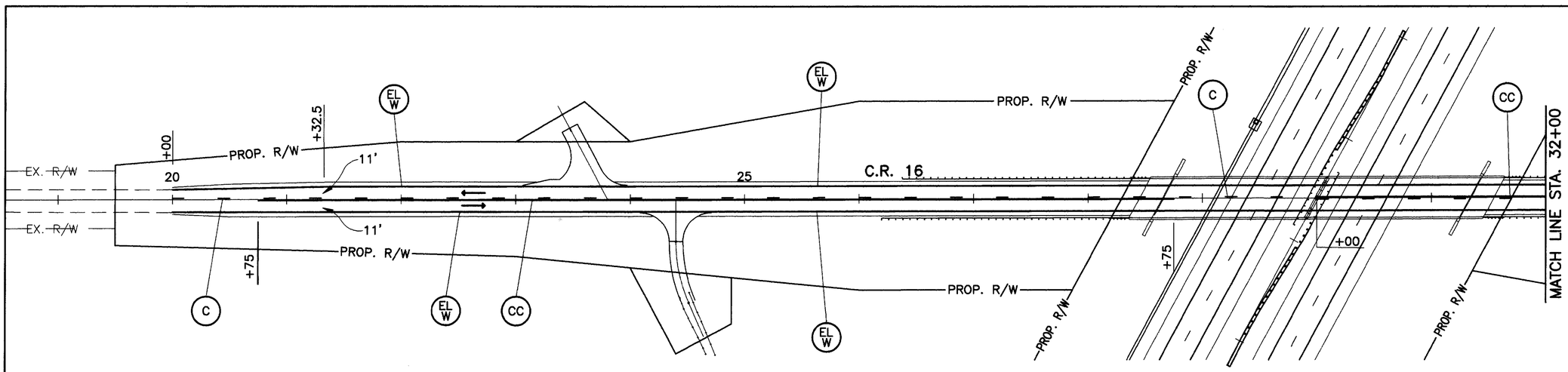
TRAFFIC CONTROL
STA. 365+00 TO STA. 395+00

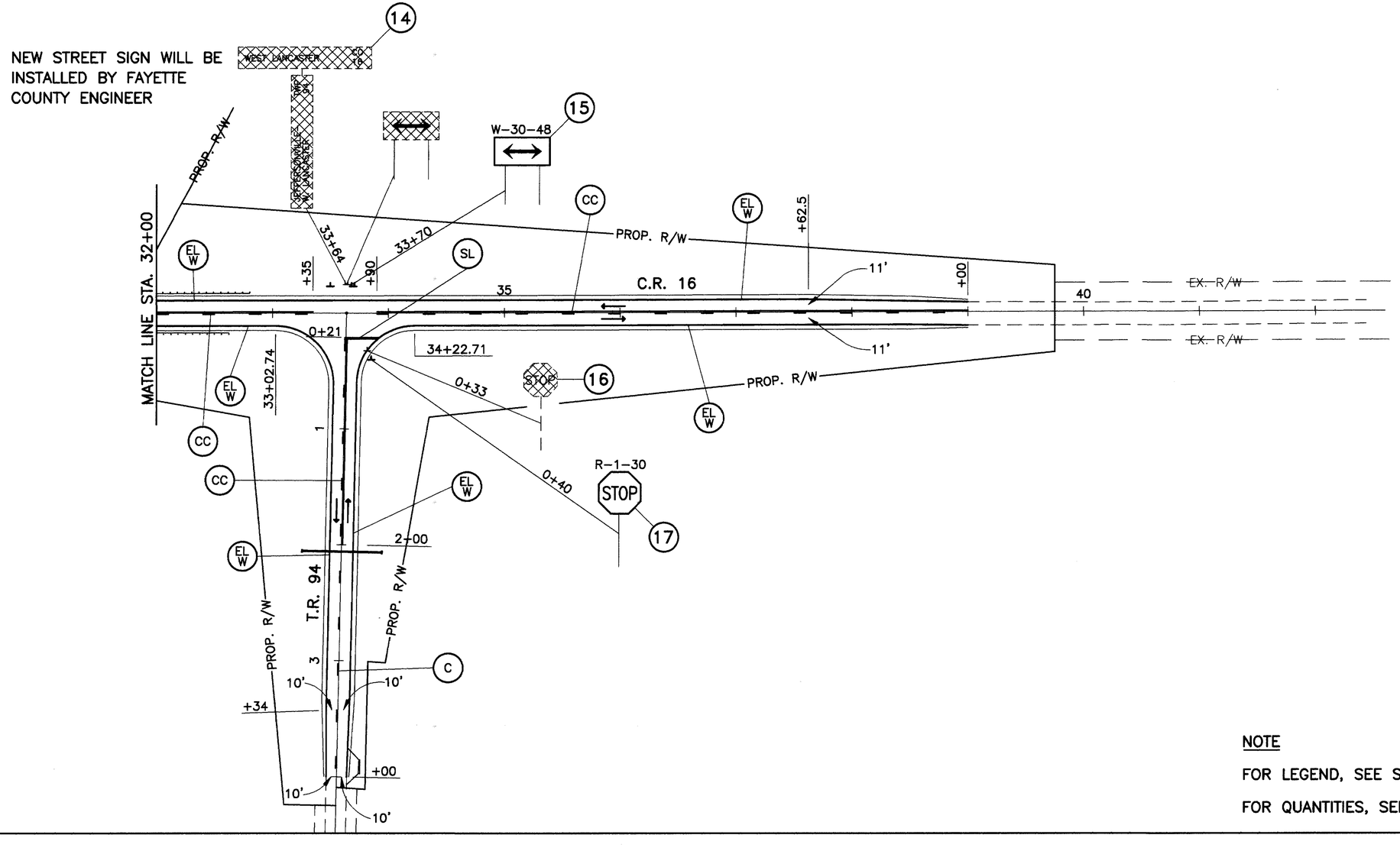
FAY-35-2.57

11 OF 30

364
522

Q:\3590\TRA_CONL\3590TC6





NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 356 & 357

N

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED M.T.
CHECKED W.G.Y.

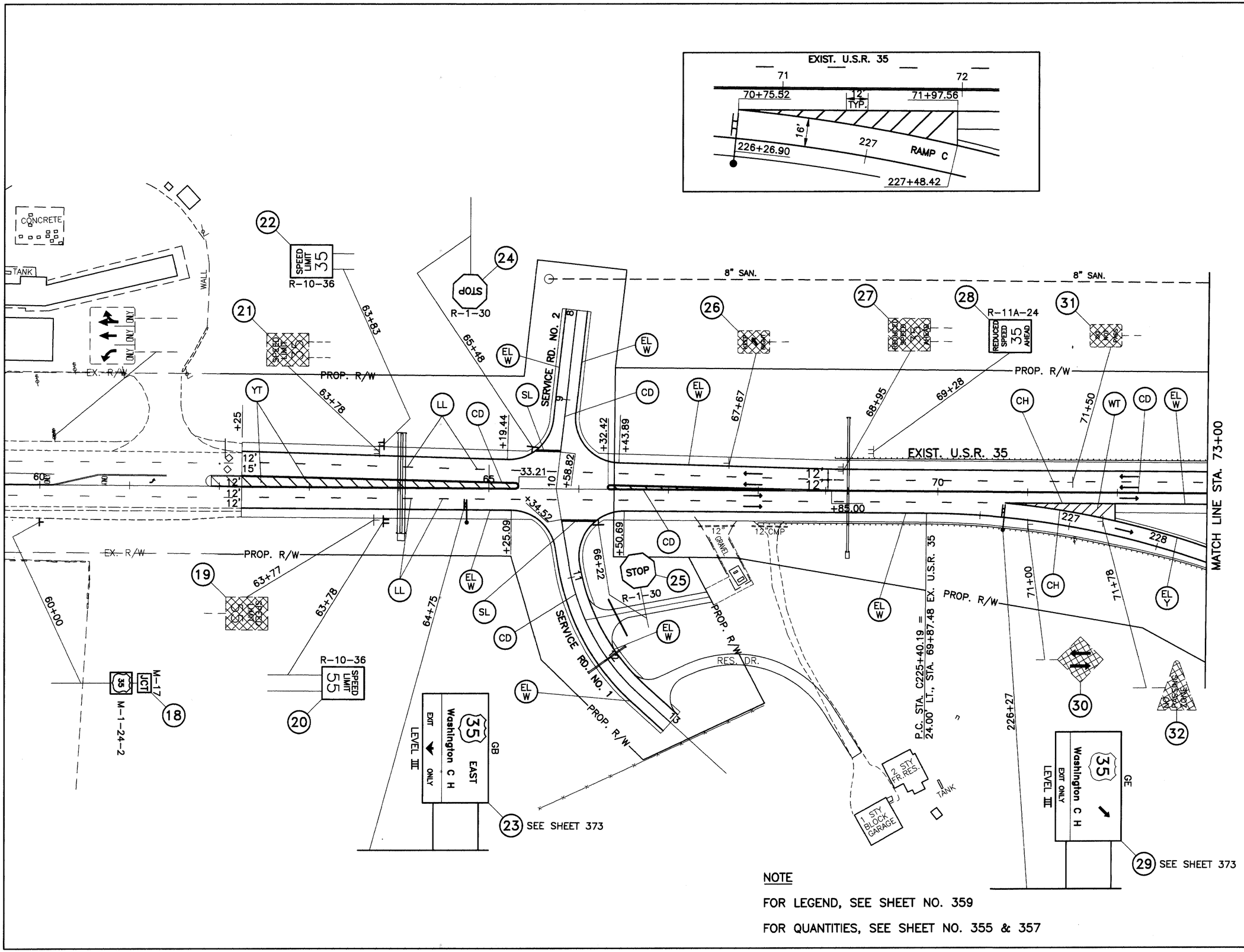
TRAFFIC CONTROL
STA. 19+00 TO STA. 42+00 - C.R. 16

FAY-35-2.57

12 OF 30

365
522

Q:\3590\TRA_CONL\3590TC7



NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355 & 357

Q:\3590\TRA_CONL\3590TC8

CALCULATED M.T.
CHECKED W.G.Y.

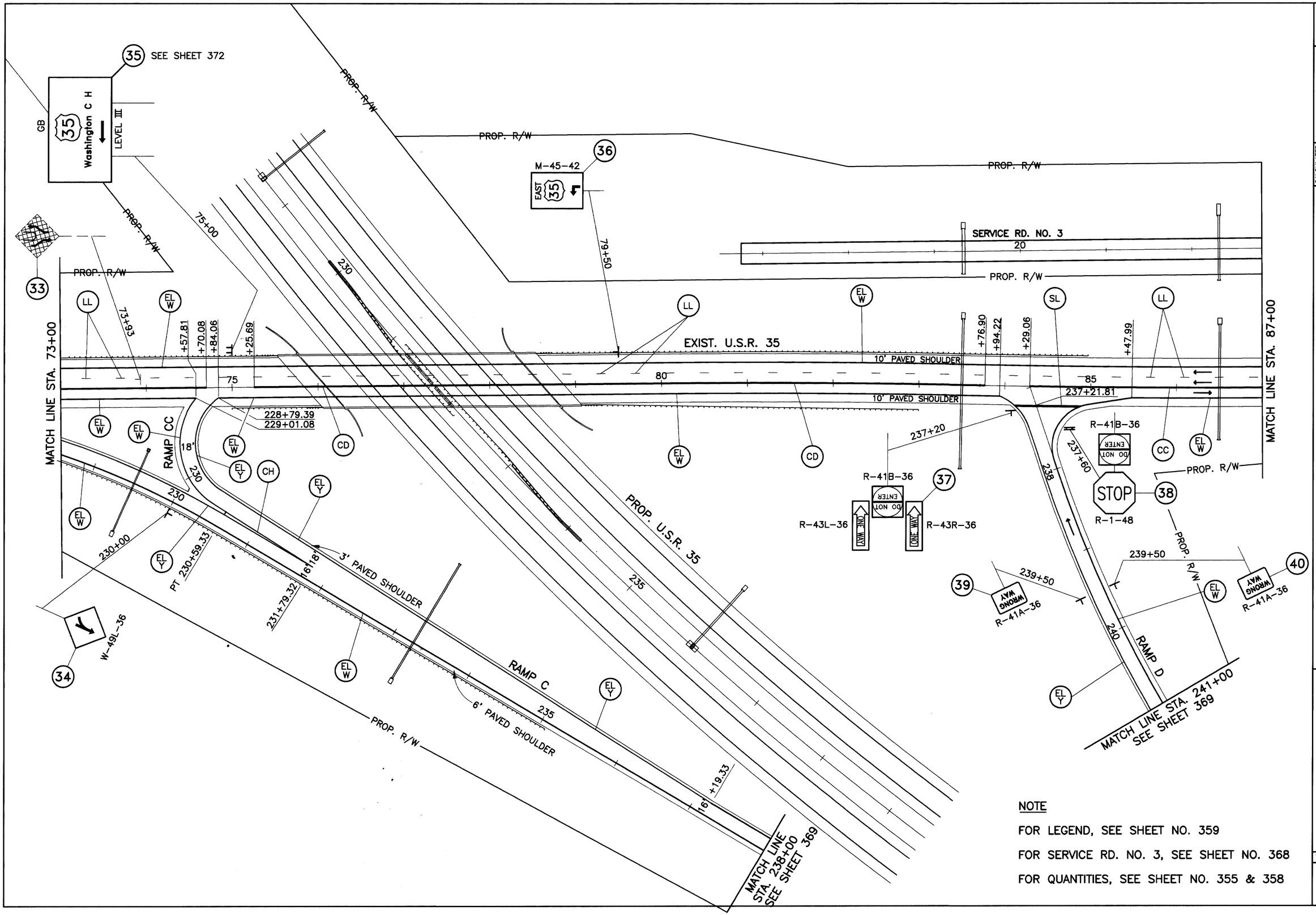
HORIZONTAL SCALE IN FEET
0 25 50 100

SIGNING & STRIPING
STA. 60+00 TO STA. 73+00 - EXIST. U.S. 35

FAY-35-2.57

13 OF 30

366
522



NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR SERVICE RD. NO. 3, SEE SHEET NO. 368

FOR QUANTITIES, SEE SHEET NO. 355 & 358

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED M.T.
CHECKED W.G.Y.

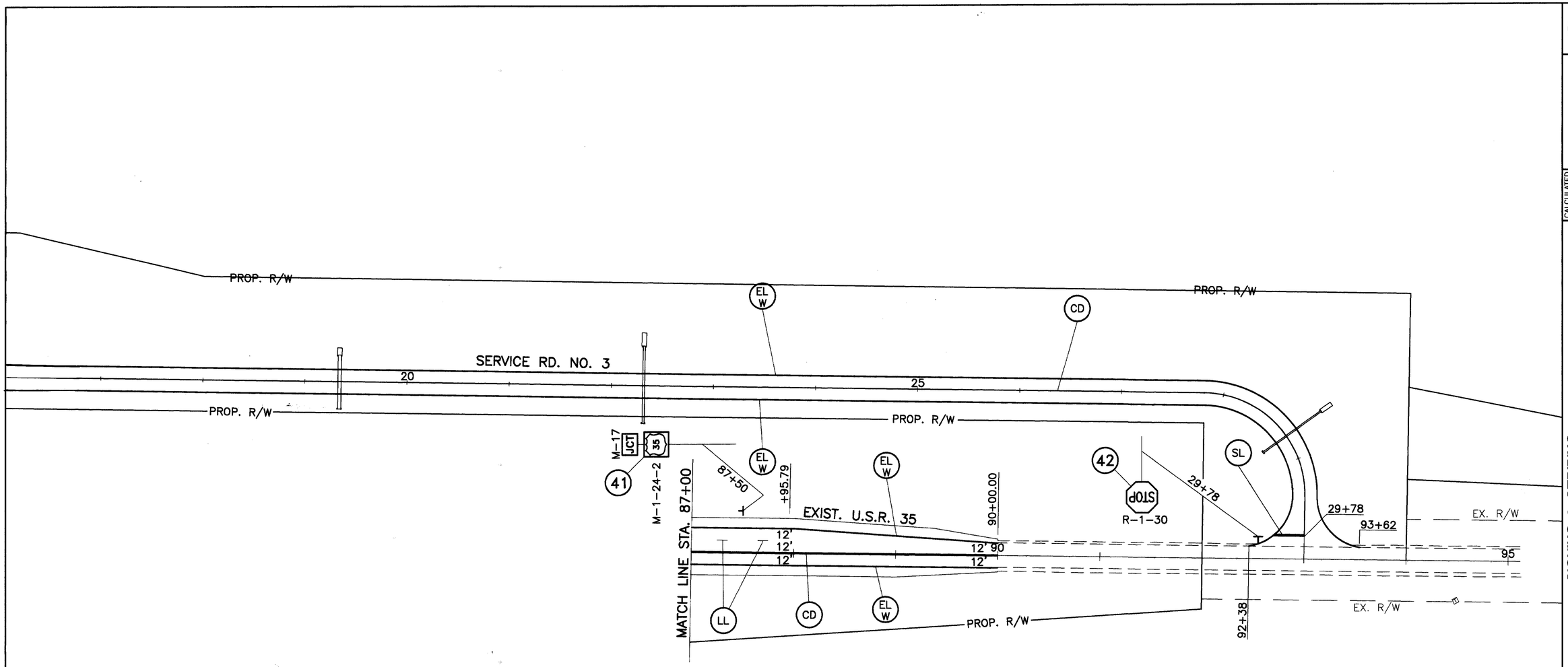
SIGNING & STRIPING
STA. 73+00 TO STA. 87+00 - EXIST. U.S. 35, RAMP C & RAMP D

FAY-35-2.57

14 OF 30

367
522

Q:\3590\TRA_CONL\3590TC9



NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355 & 358

CALCULATED M.T.
CHECKED W.G.Y.

0 25 50 100
HORIZONTAL SCALE IN FEET

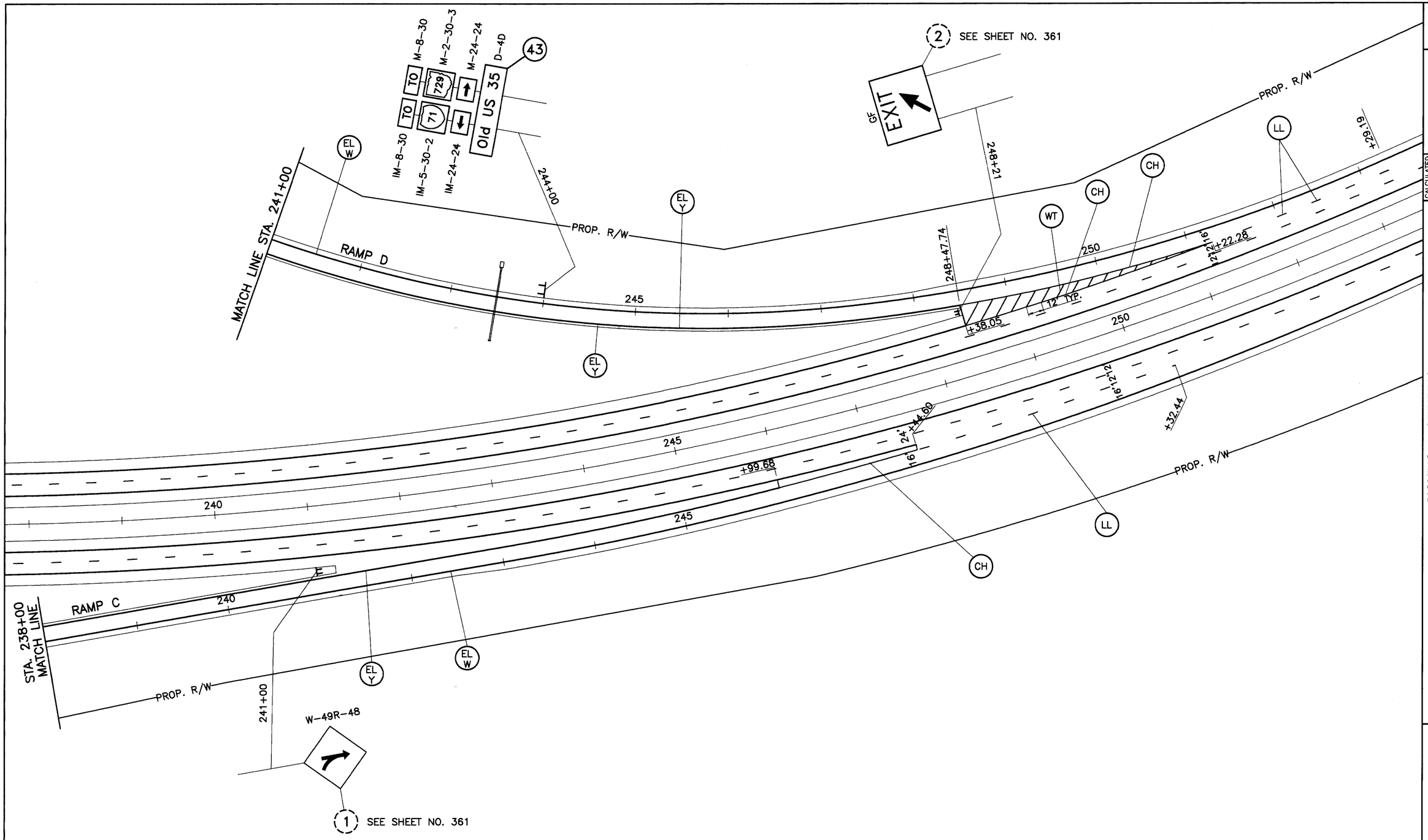
SIGNING & STRIPING
STA. 87+00 TO STA. 90+00 EXISTING USR 35
STA. 22+00 TO STA. 30+00 SERVICE RD. #3

FAY-35-2.57

15 OF 30

368
522

Q:\3590\TRA_CONL\3590TC10



NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 355 & 358

CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

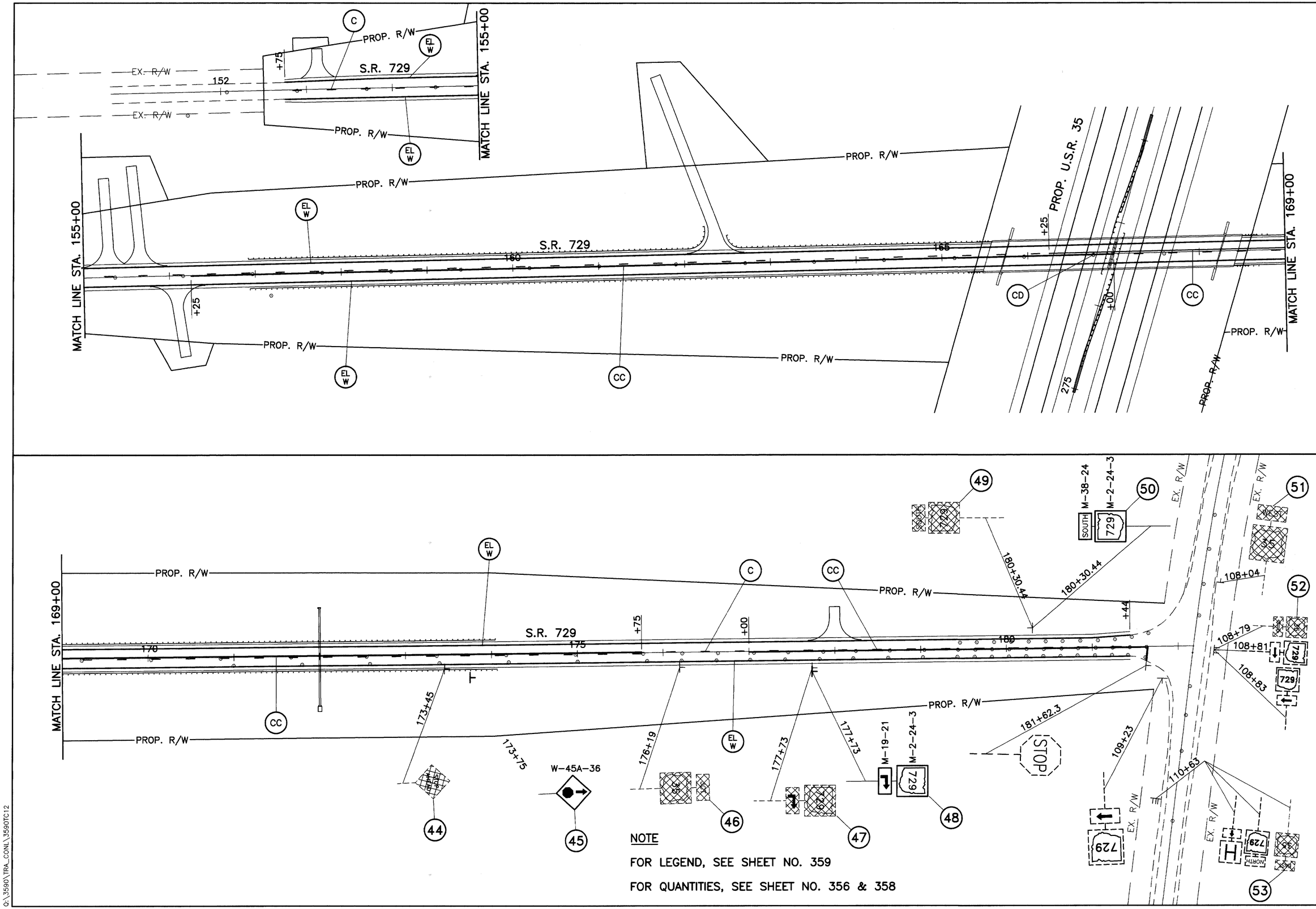
SIGNING & STRIPING
STA. 238+00 TO STA. 253+00 - RAMP C
STA. 241+00 TO STA. 253+00 - RAMP D

FAY-35-2.57

16 of 30

369
522

Q:\3590\TRA_CONL\3590TC11



NOTE

FOR LEGEND, SEE SHEET NO. 359

FOR QUANTITIES, SEE SHEET NO. 356 & 358

CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

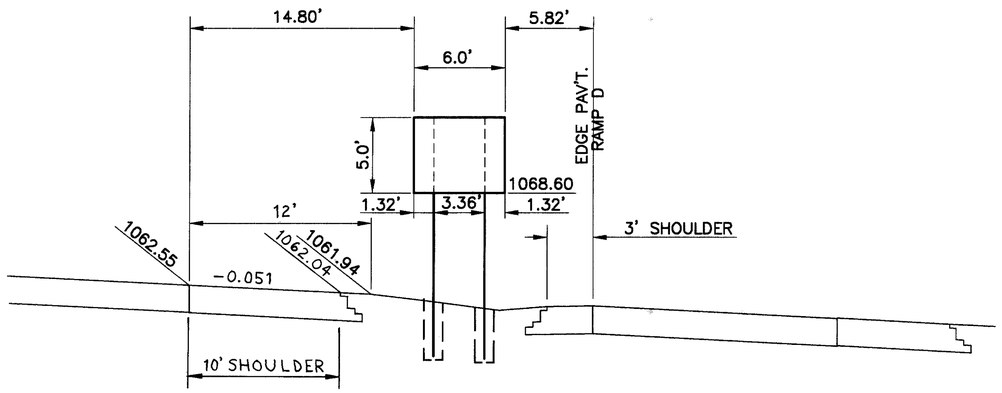
TRAFFIC CONTROL
STA. 152+00 TO STA. 182+00 - S.R. 729

FAY-35-2.57

17 OF 30

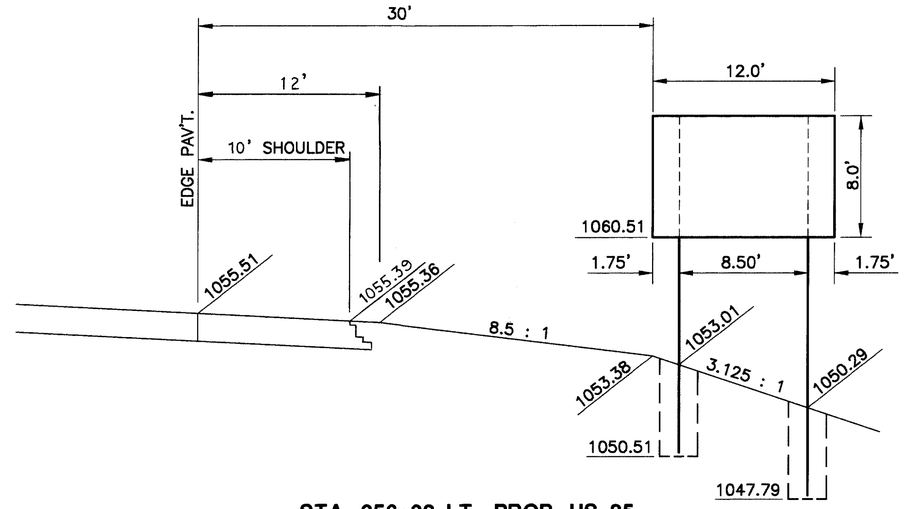
370
522

Q:\3590\TRA_CONL\3590TC12



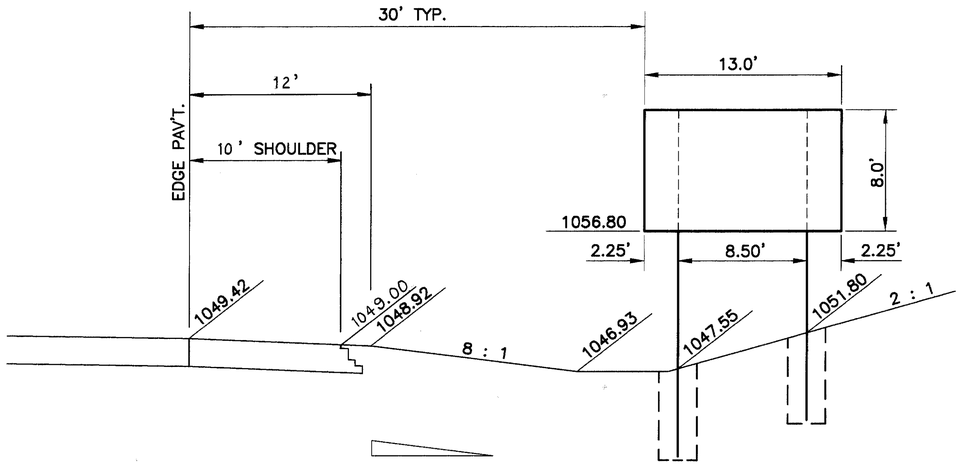
STA. 248+08 LT., PROP. US 35

SIGN NO. 2



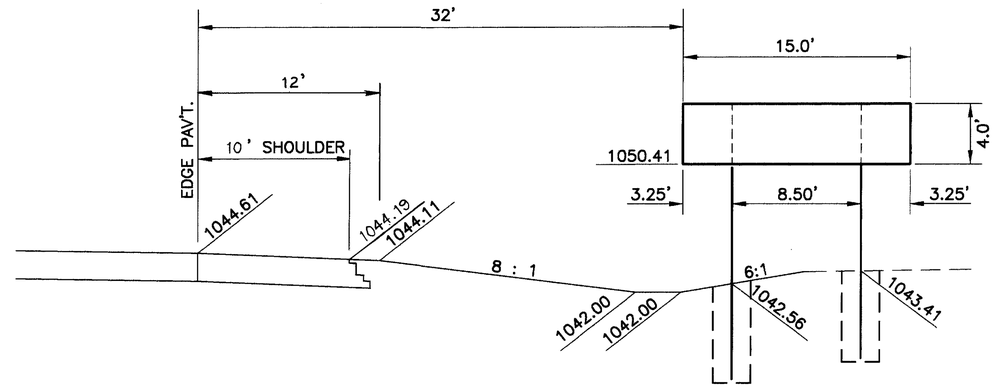
STA. 256+38 LT., PROP. US 35

SIGN NO. 3



STA. 273+60, PROP. US 35

SIGN NO. 6



STA. 295+00 RT., PROP. US 35

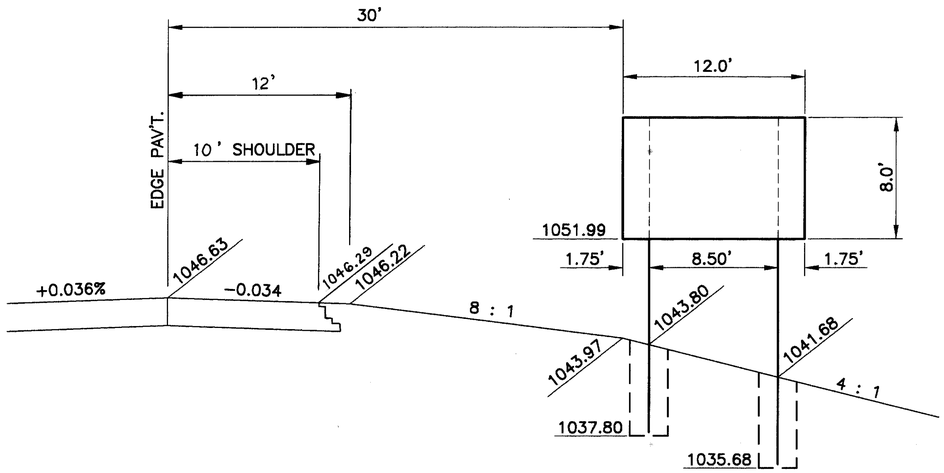
SIGN NO. 9

REVISED	CALCULATED
10-99	J.L.G.
BY	CHECKED
F Y C	W.G.Y.

SIGNING DETAILS

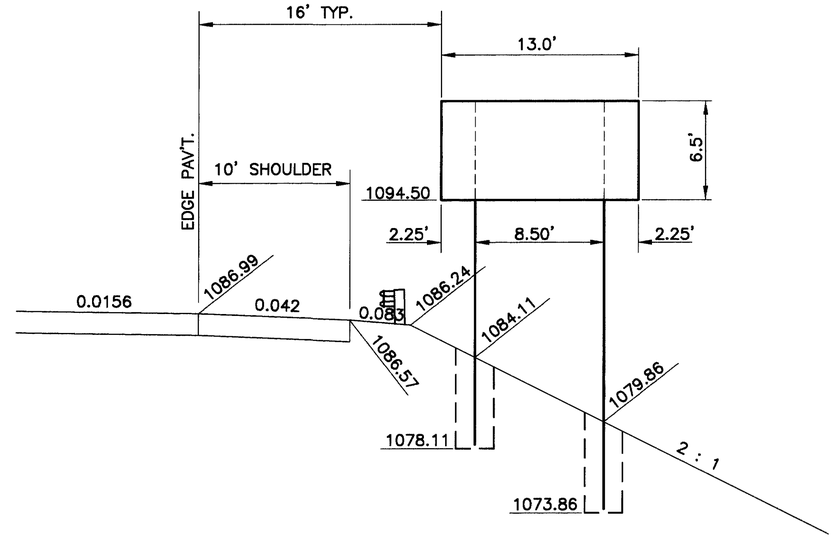
FAY-35-2.57

Q:\3590\TRA_CONL\3590SD1



STA. 301+00, PROP. US 35

SIGN NO. 10



STA. 75+00, EX. US 35

SIGN NO. 35

REVISED	CALCULATED
10-99	J.L.G.
BY	CHECKED
F Y C	W.G.Y.

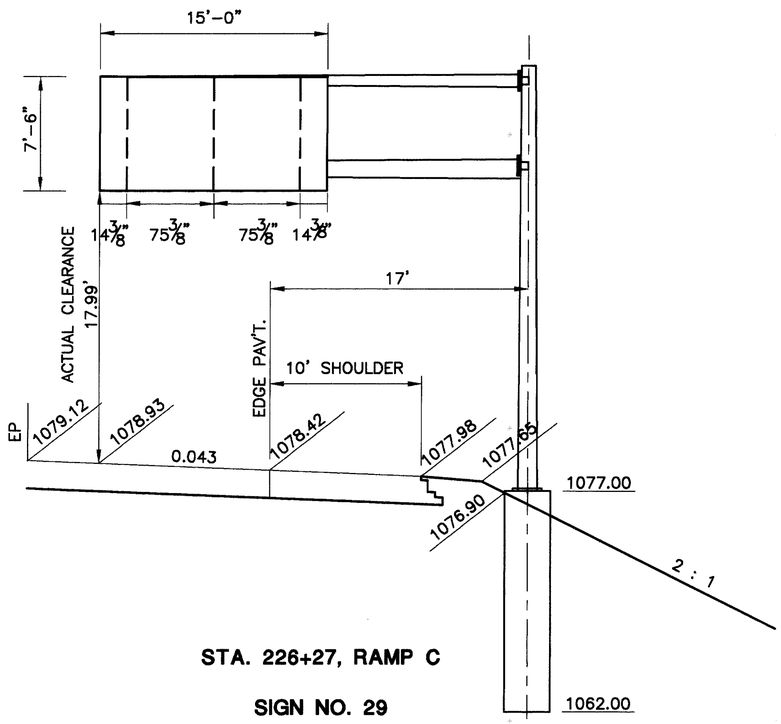
SIGNING DETAILS

FAY-35-2.57

19 OF 30

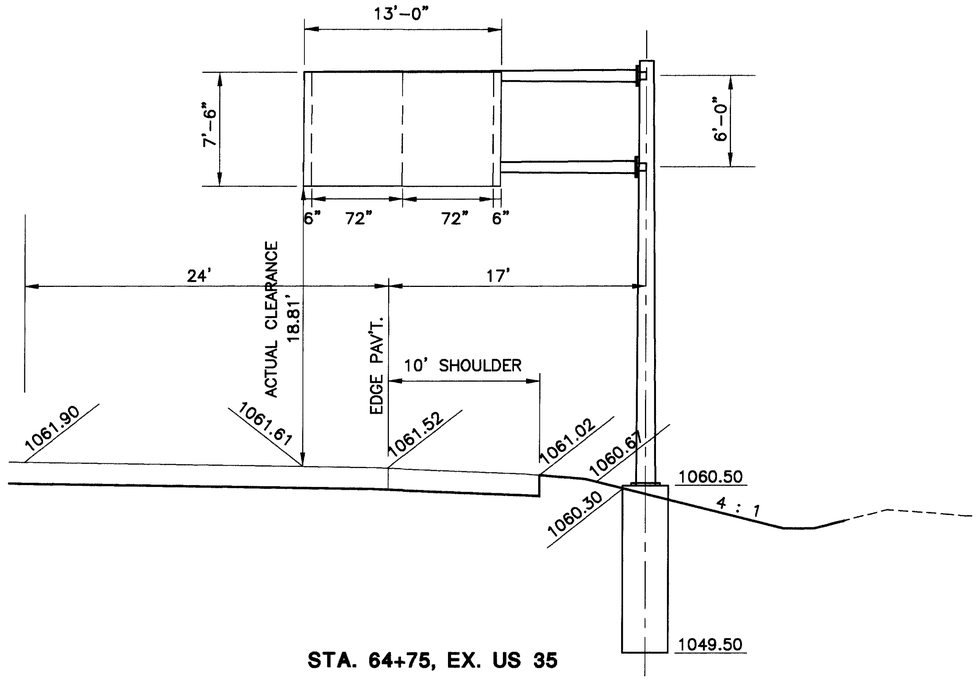
372
522

Q:\3590\TRA_CONL\3590SD3



STA. 226+27, RAMP C

SIGN NO. 29



STA. 64+75, EX. US 35

SIGN NO. 23

SIGN LOCATION	SIDE	CODE	EFFECTIVE SIGN SIZE	POST LOCATE OFFSET FOR EP	630						631			625
					TYPE G EXTRUSHEET SIGNS	RIGID OVERHEAD SIGN SUPPORT FOUNDATION	TC-12.30 SUPPORT OVERHEAD SIGN DESIGN 5	TC-12.30 SUPPORT OVERHEAD SIGN DESIGN 8	SIGN SERVICE	SIGN WIRED	TYPE X W/ENCLOSURE DISCONNECT SWITCH	REMOTE CMRI-175-480, BALLAST, TYPE	WATT LAMP TC-31.21 W/175 LUMINAIRE, TYPE MERCURY VAPOR		GROUND ROD
					SQ. FT.	EACH	22	28	EACH	EACH	EACH	EACH	EACH		EACH
64+75, EX. U.S. 35	RT	GB	13' X 6.5'	17'	84.5	1	1		1	1	1	2	2		1
226+27, RAMP C	RT	GE	15' X 6.5'	17'	97.5	1		1	1	1	1	2	2		1
TOTALS					182	2	1	1	2	2	2	4	4		2

CALCULATED J.L.G.
CHECKED W.G.Y.

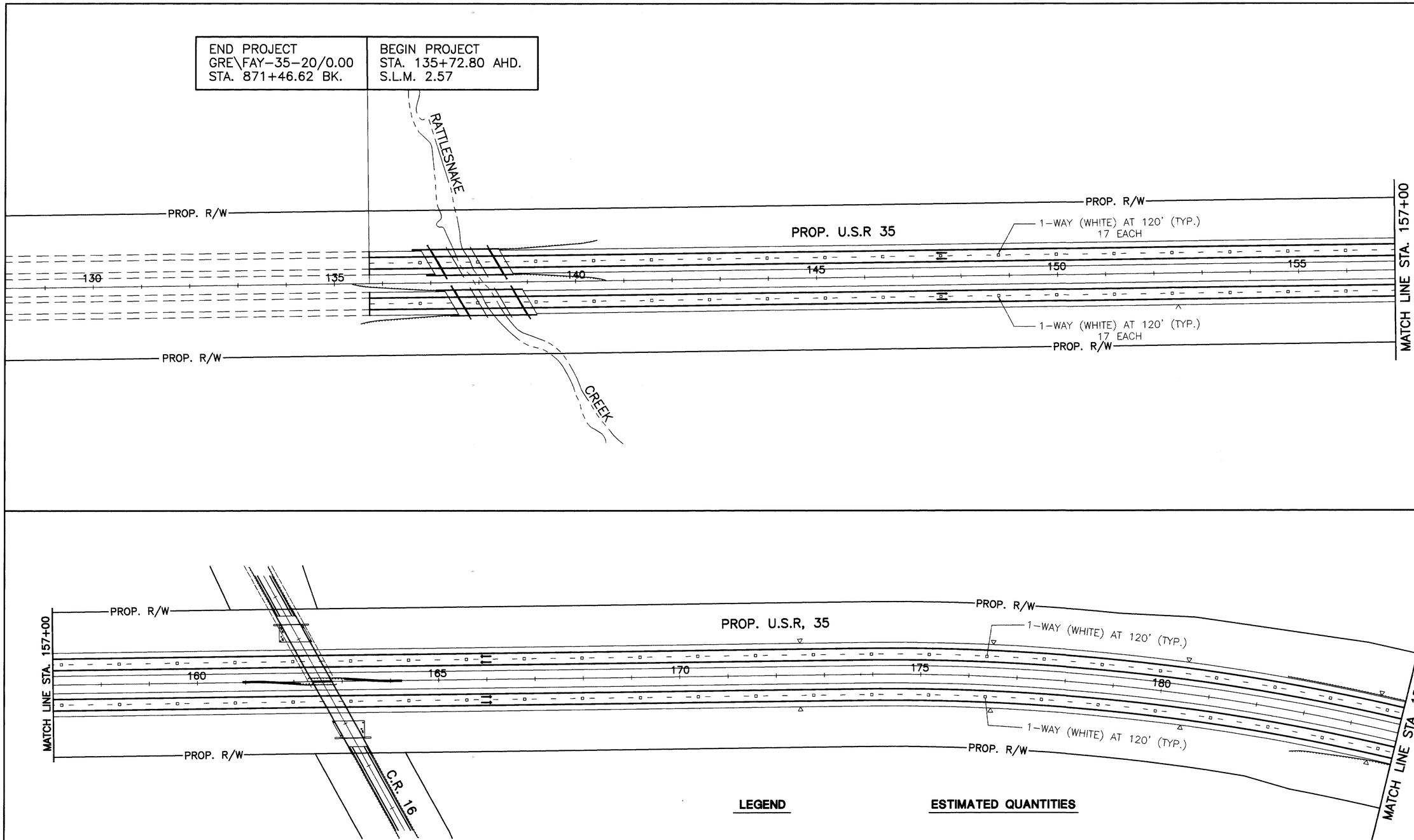
SIGNING DETAILS

FAY-35-2.57

20 OF 30

373
522

Q:\3590\TRA_CONL\3590SD2



LEGEND

- □ 1-WAY RAISED PAVEMENT MARKER
- ○ 2-WAY RAISED PAVEMENT MARKER
- △ DELINEATOR TYPE C
- ▲ DELINEATOR TYPE D

ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS	
ITEM 621 1-WAY (WHITE)	82 EA.
DELINEATORS	
ITEM 620 TYPE C, POST MOUNTED	8 EA.

NOTES

FOR QUANTITIES, SEE SHEET NO. 356

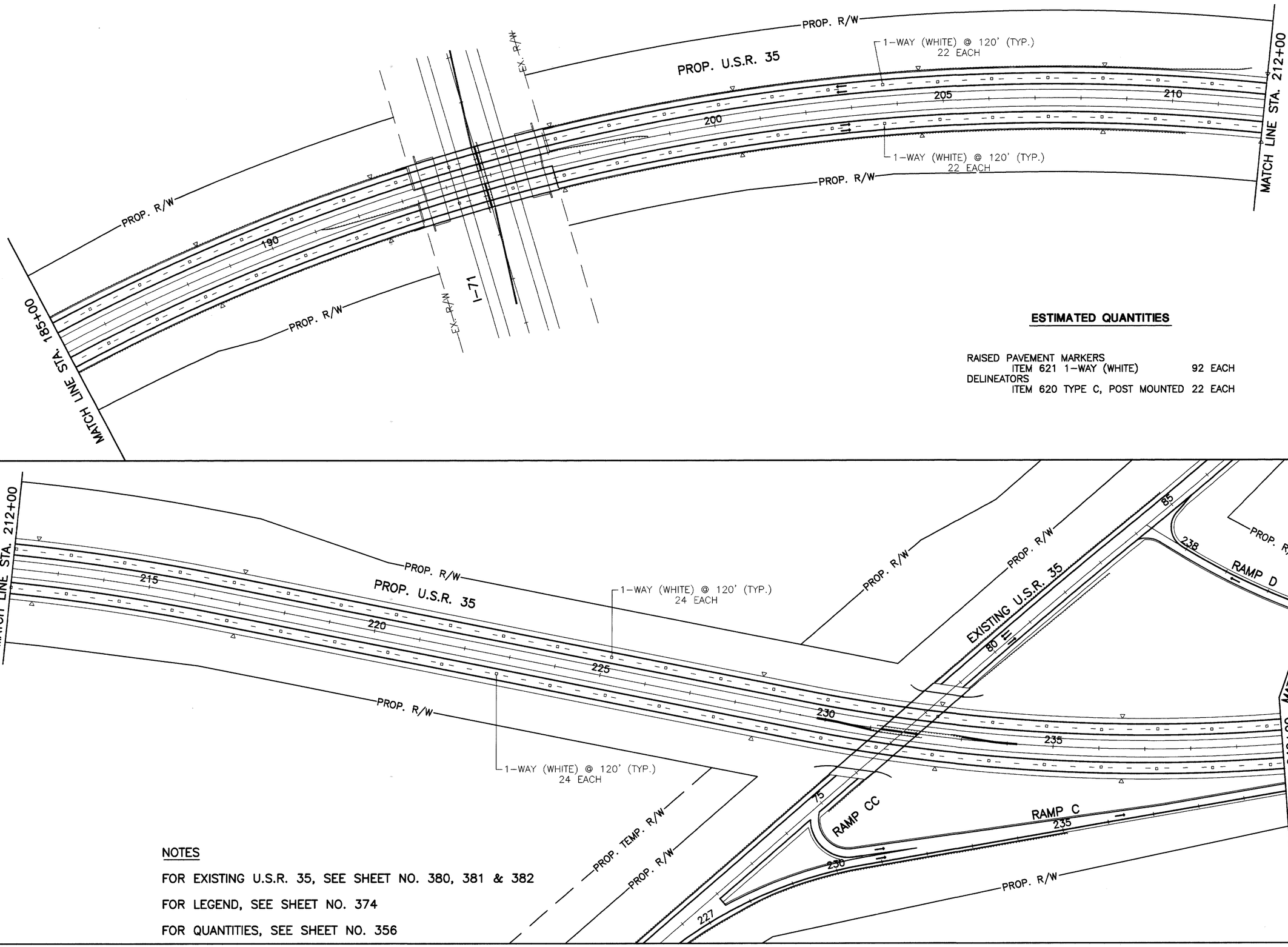
CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 50 100 200

RAISED PAVEMENT MARKERS & DELINEATOR PLAN
STA. 130+00 TO STA. 185+00

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC13



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS
ITEM 621 1-WAY (WHITE) 92 EACH
DELINEATORS
ITEM 620 TYPE C, POST MOUNTED 22 EACH

NOTES

FOR EXISTING U.S.R. 35, SEE SHEET NO. 380, 381 & 382

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

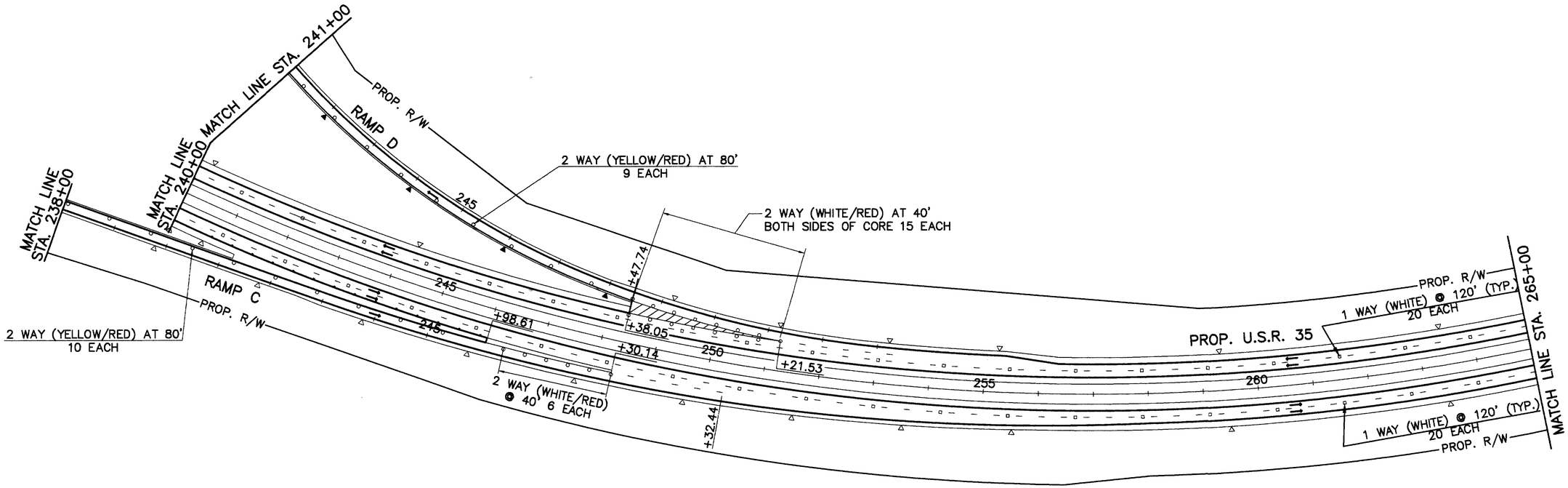
HORIZONTAL SCALE IN FEET
0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

RAISED PAVEMENT MARKER & DELINEATOR PLAN
STA. 185+00 TO STA. 240+00

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC14



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS

ITEM 621 1 WAY (WHITE)	40 EACH
ITEM 621 2 WAY (YELLOW/RED)	19 EACH
ITEM 621 2 WAY (WHITE/RED)	21 EACH

DELINEATORS

ITEM 620 TYPE C, POST MOUNTED	19 EACH
ITEM 620 TYPE D, POST MOUNTED	4 EACH

NOTES

1. FOR LEGEND, SEE SHEET NO. 374
2. FOR QUANTITIES, SEE SHEET NO. 356

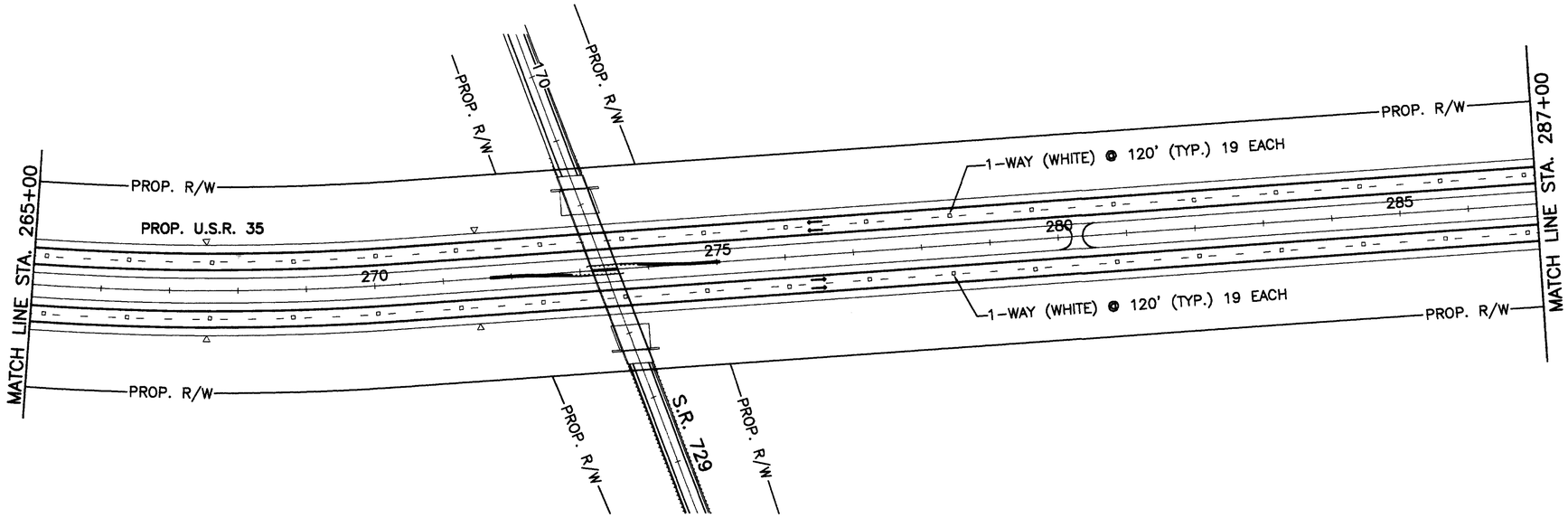
CALCULATED M.T.
CHECKED W.G.Y.

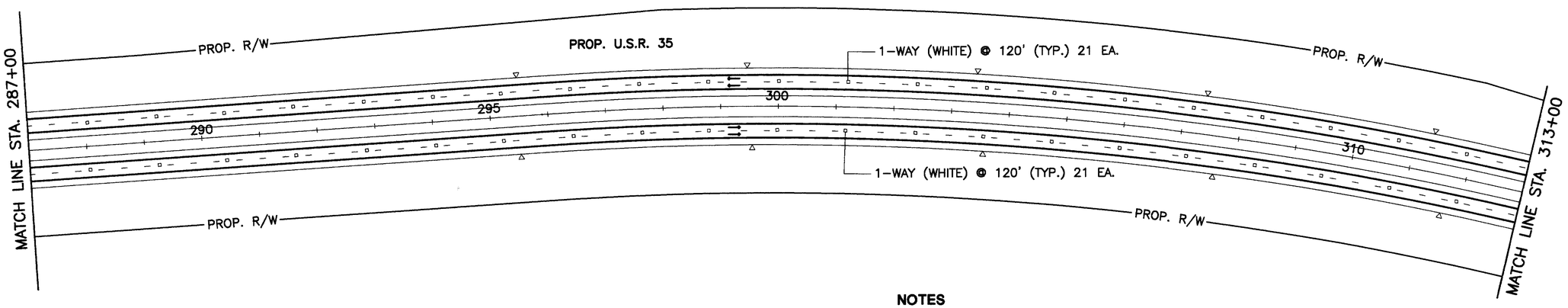
0 50 100 200
HORIZONTAL SCALE IN FEET

RAISED PAVEMENT MARKER & DELINEATOR PLAN STA. 240+00 TO STA. 265+00

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC15





ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS	
ITEM 621 1-WAY (WHITE)	80 EACH
DELINEATORS	
ITEM 620 TYPE C, POST MOUNTED	14 EACH

NOTES

FOR LEGEND, SEE SHEET NO. 374

FOR S.R. 729, SEE SHEET NO. 383

FOR QUANTITIES, SEE SHEET NO. 356

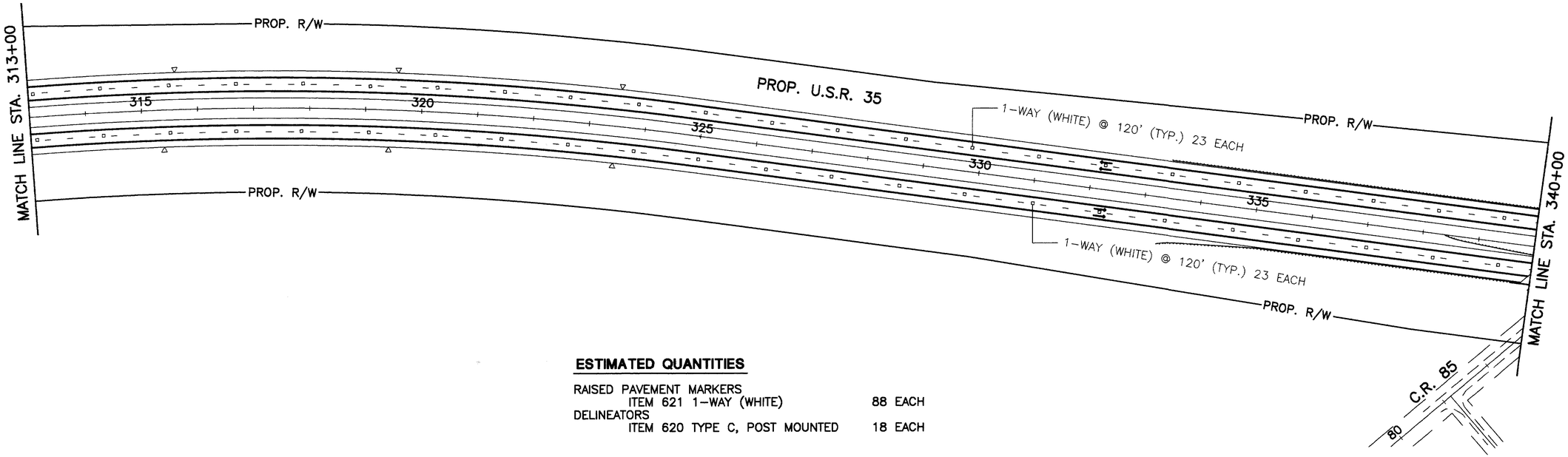
SCALE IN FEET 0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

RAISED PAVEMENT MARKER & DELINEATOR PLAN STA. 265+00 TO STA. 313+00

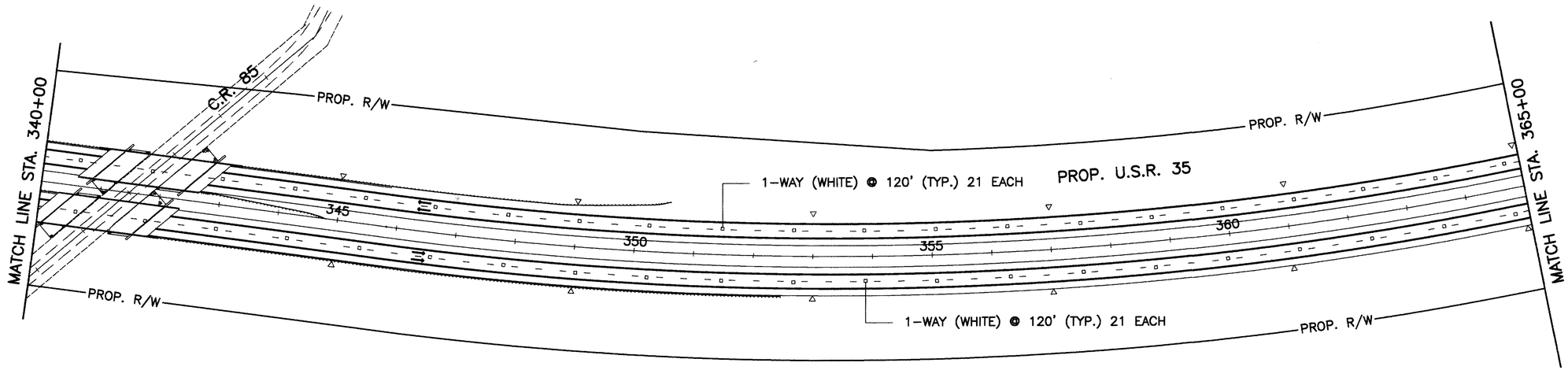
FAY-35-2.57

Q:\3590\TRA_CONL\3590TC16



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS	
ITEM 621 1-WAY (WHITE)	88 EACH
DELINEATORS	
ITEM 620 TYPE C, POST MOUNTED	18 EACH



NOTES

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

HORIZONTAL SCALE IN FEET
0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

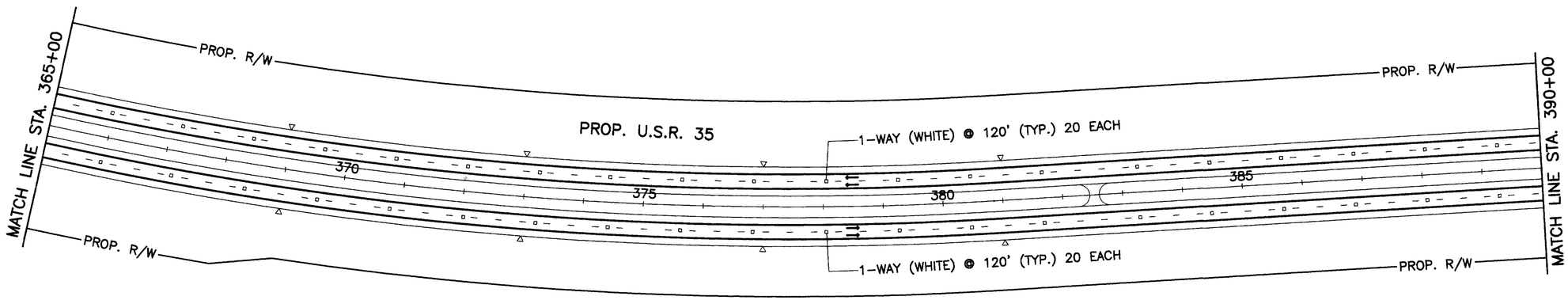
RAISED PAVEMENT MARKER & DELINEATOR PLAN STA. 313+00 TO STA. 365+00

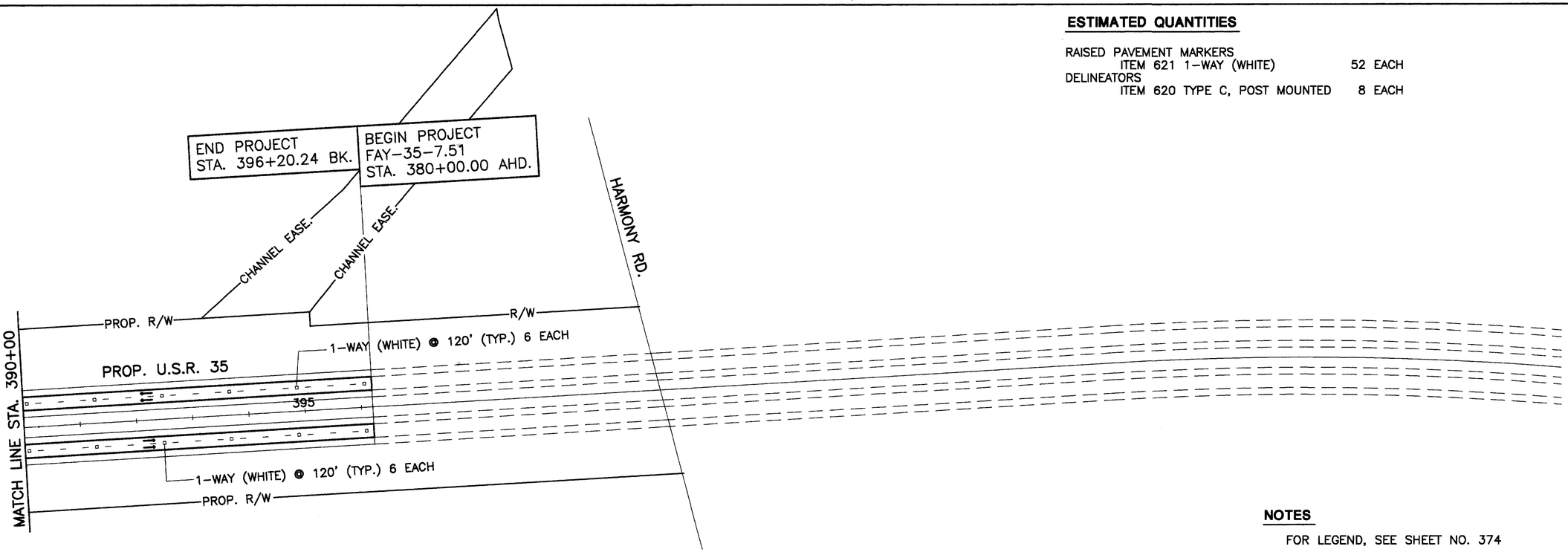
FAY-35-2.57

25 OF 30

378
522

Q:\3590\TRA_CONL\3590TC17





ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS	
ITEM 621 1-WAY (WHITE)	52 EACH
DELINEATORS	
ITEM 620 TYPE C, POST MOUNTED	8 EACH

NOTES

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

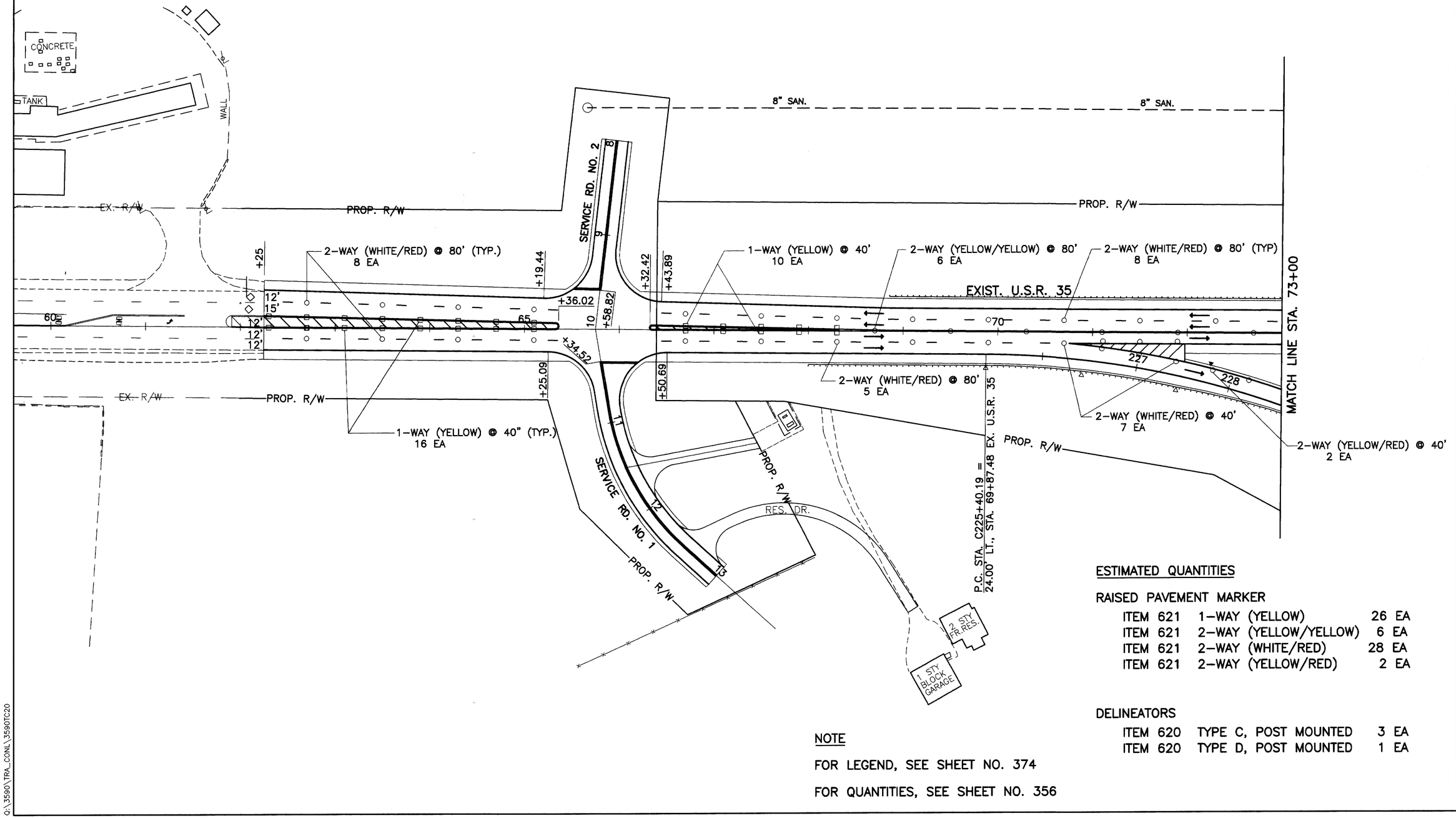
HORIZONTAL SCALE IN FEET
0 50 100 200

CALCULATED M.T.
CHECKED W.G.Y.

RAISED PAVEMENT MARKERS & DELINEATOR PLAN
STA. 365+00 TO STA. 395+00

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC18



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKER

ITEM 621	1-WAY (YELLOW)	26	EA
ITEM 621	2-WAY (YELLOW/YELLOW)	6	EA
ITEM 621	2-WAY (WHITE/RED)	28	EA
ITEM 621	2-WAY (YELLOW/RED)	2	EA

DELINEATORS

ITEM 620	TYPE C, POST MOUNTED	3	EA
ITEM 620	TYPE D, POST MOUNTED	1	EA

NOTE

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

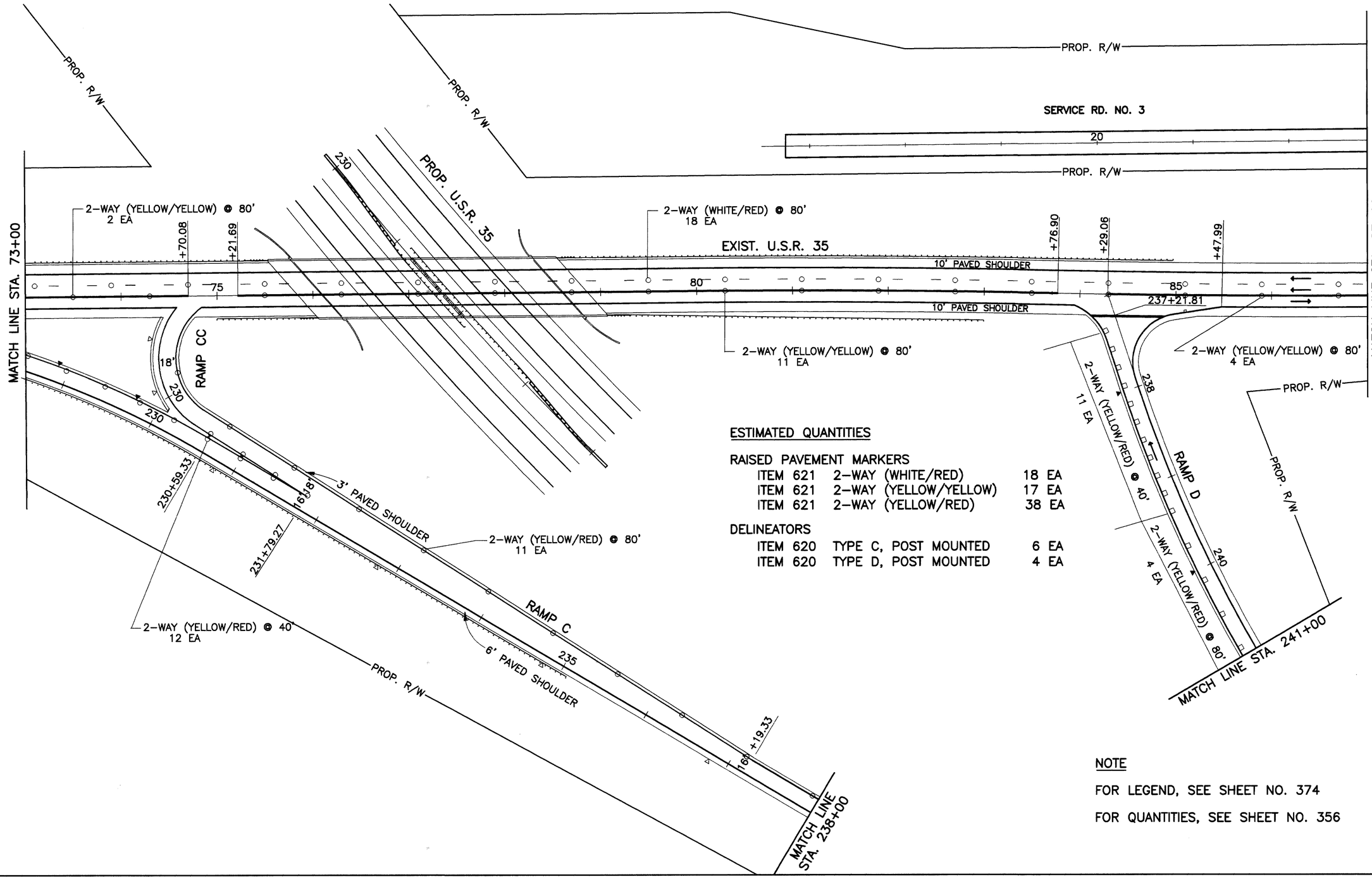
CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

RAISED PAVEMENT MARKER & DELINEATOR PLAN
STA. 60+00 TO STA. 73+00 - EXIST. U.S. 35

FAY-35-2.57

Q:\3590\TRA_CONL\3590TC20



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS

ITEM 621	2-WAY (WHITE/RED)	18 EA
ITEM 621	2-WAY (YELLOW/YELLOW)	17 EA
ITEM 621	2-WAY (YELLOW/RED)	38 EA

DELINEATORS

ITEM 620	TYPE C, POST MOUNTED	6 EA
ITEM 620	TYPE D, POST MOUNTED	4 EA

NOTE

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

HORIZONTAL SCALE IN FEET
0 25 50 100

CALCULATED M.T.
CHECKED W.G.Y.

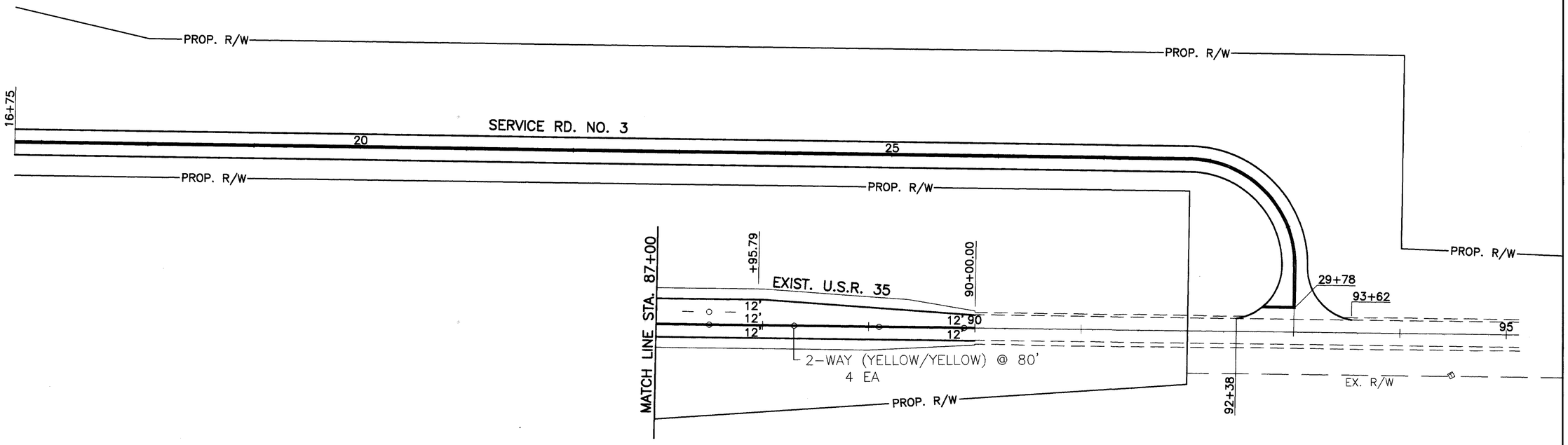
RAISED PAVEMENT MARKER & DELINEATOR PLAN
STA. 73+00 TO STA. 87+00 - EXIST. U.S. 35, RAMP C & RAMP D

FAY-35-2.57

28 OF 30

381
522

Q:\3590\TRA_CONL\3590TC21



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS
ITEM 621 2-WAY (YELLOW/YELLOW) 4 EA

NOTE

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

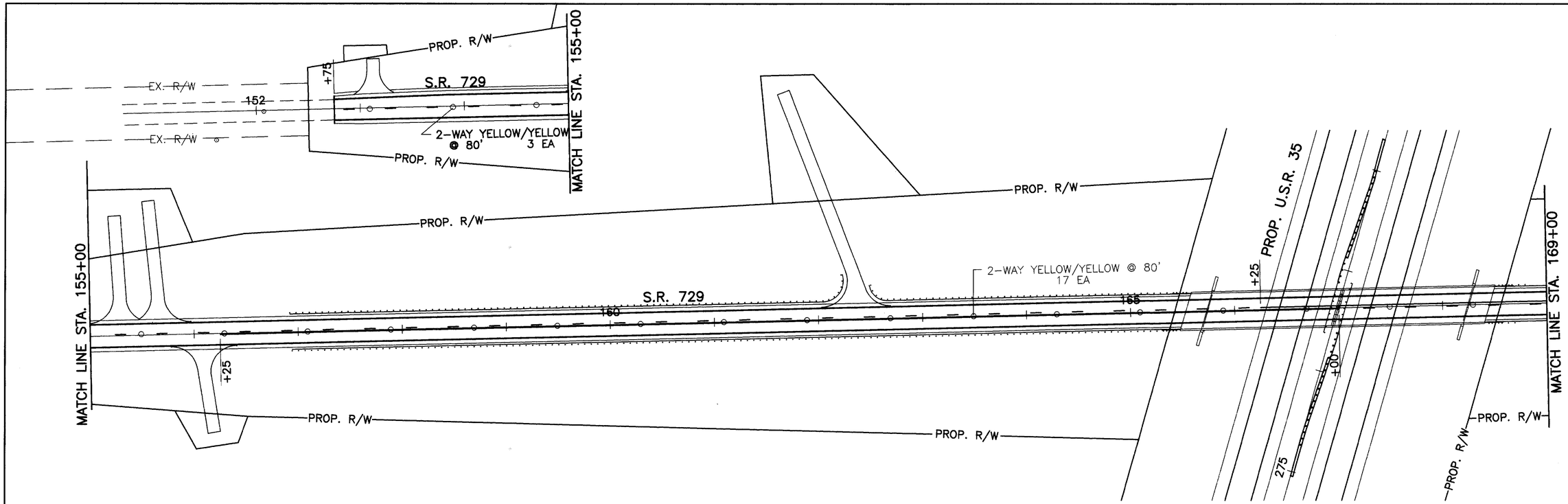
CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

RAISED PAVEMENT MARKERS & DELINEATOR PLAN
STA. 87+00 TO STA. 95+00 EXISTING U.S.R. 35

FAY-35-2.57

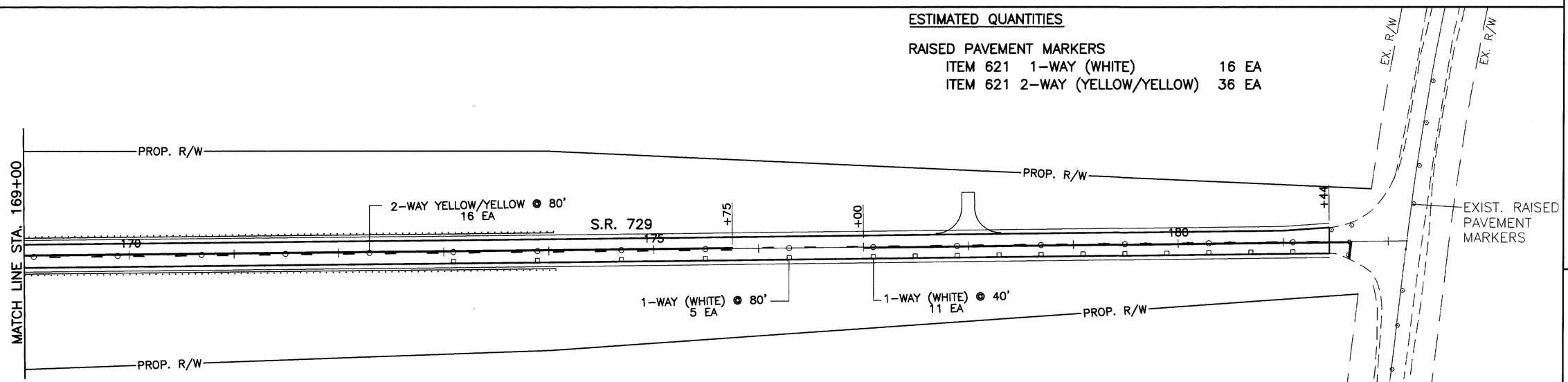
Q:\3590\TRA_CONL\3590TC22



ESTIMATED QUANTITIES

RAISED PAVEMENT MARKERS

ITEM 621	1-WAY (WHITE)	16 EA
ITEM 621	2-WAY (YELLOW/YELLOW)	36 EA



NOTE

FOR LEGEND, SEE SHEET NO. 374

FOR QUANTITIES, SEE SHEET NO. 356

CALCULATED M.T.
CHECKED W.G.Y.

HORIZONTAL SCALE IN FEET
0 25 50 100

RAISED PAVEMENT MARKER & DELINEATOR PLAN
STA. 152+00 TO STA. 182+00 - S.R. 729

FAY-35-2.57

30 OF 30

383
522

Q:\3590\TRA_CONL\3590TC23

CALCULATED EAH 7/96
CHECKED SAW 9/96

LIGHTING GENERAL NOTES

625.03 – GENERAL

THE POWER SUPPLYING AGENCY FOR THIS PROJECT IS:

DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURTHOUSE PLAZA, SW
DAYTON, OHIO 45401
(513) 331–4498

THIS PROJECT HAS BEEN DESIGNED ON THE BASIS OF 5% VOLTAGE DROP WITH A MAXIMUM UNIFORMITY RATIO OF 4.0 TO 1 FOR CONVENTIONAL UNITS.

UTILITIES NOTIFICATION

AT LEAST TWO (2) WORKING DAYS PRIOR TO COMMENCING CONSTRUCTION OPERATIONS IN ANY AREA WHICH MAY INVOLVE UNDERGROUND FACILITIES THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER, THE REGISTERED UNDERGROUND UTILITY PROTECTION SERVICES AND THE OWNERS OF ALL UNDERGROUND UTILITY FACILITES SHOWN IN THE PLANS.

AFTER NOTIFICATION THE OWNER OF ANY UNDERGROUND UTILITY FACILITY THAT IS TO REMAIN IN SERVICE DURING AND OR AFTER CONSTRUCTION SHALL WITHIN FORTY–EIGHT (48) HOURS, EXCLUDING SATURDAYS, SUNDAYS AND LEGAL HOLIDAYS, STAKE MARK OR OTHERWISE DESIGNATE THE LOCATION OF THE UNDERGROUND FACILITIES IN THE CONSTRUCTION AREA IN SUCH A MANNER AS TO INDICATE THEIR COURSE TOGETHER WITH THE APPROXIMATE DEPTH AT WHICH THEY WERE INSTALLED. THE MARKING OR LOCATING SHALL BE COORDINATED TO STAY APPROXIMATELY TWO (2) DAYS AHEAD OF THE PLANNED CONSTRUCTION.

713.14 LAMPS

HIGH PRESSURE SODIUM LAMPS SHALL BE GENERAL ELECTRIC "LUCALOX", WESTINGHOUSE "CERMALUX", SYLVANIA "LUMALUX", OR EQUAL APPROVED BY THE ENGINEER.

UNDERDRAINS FOR PULL BOXES

REFERENCE IS MADE TO STANDARD DRAWING HL–30.11 FOR DETAILS OF DRAINING PULL BOXES. UNDERDRAINS FOR PULL BOXES SHALL BE USED AS DIRECTED BY THE ENGINEER AND SHALL BE PROVIDED WHERE THE LENGTH REQUIRED FOR A SATISFACTORY OUTLET DOES NOT EXCEED APPROXIMATELY 20 FEET. AN ESTIMATED QUANTITY OF "140 LINEAR FEET OF ITEM 603, 4" CONDUIT TYPE E IS INCLUDED IN THE LIGHTING GENERAL SUMMARY FOR THIS PURPOSE.

LIGHT TOWER LUMINAIRE MOUNTING ARRANGEMENT

LUMINAIRE MOUNTING ARMS FOR TOWER LIGHTING UNITS SHALL BE INSTALLED BY THE POLE MANUFACTURER SO THAT THE REQUIRED NUMBER OF LUMINAIRES CAN BE INSTALLED ON THE LUMINAIRE MOUNTING RING IN A SYMMETRICAL ARRANGEMENT. WHEN ONLY TWO ARMS ARE REQUIRED THEY SHALL BE POSITIONED SO THE ARMS ARE PARALLEL TO THE CENTERLINE OR BASELINE OF THE PAVEMENT FROM WHICH THE TOWER IS STATIONED.

UNLESS OTHERWISE SPECIFIED IN THE PLANS, ALL LUMINAIRES WITH ASYMMETRIC DISTRIBUTIONS SHALL BE INSTALLED SO THE "ARROW" OR "STREET SIDE" DESIGNATION ON THE OPTICAL ASSEMBLY IS POSITIONED PERPENDICULAR TO THE CENTERLINE OR BASELINE OF THE PAVEMENT FROM WHICH THE TOWER IS STATIONED. ANY OPTICAL ROTATION CALLED FOR WILL BE EXPRESSED AS A CLOCKWISE (CW) OR COUNTERCLOCKWISE (CCW) ANGULAR MEASUREMENT FROM THE NORMAL "ARROW" ORIENTATION.

LIGHT TOWER HANDHOLE LOCATION

FOR LIGHT TOWERS WITH MAINTENANCE PLATFORMS, THE POLE HANDHOLE SHALL BE LOCATED ON THE DOWN SLOPE OR OPEN SIDE OF THE PLATFORM. FOR LIGHT TOWERS WITHOUT MAINTENANCE PLATFORMS, THE POLE HANDHOLE SHALL BE LOCATED ON THE SIDE OPPOSITE TRAFFIC FLOW ON THE ROADWAY FROM WHICH THE TOWER IS STATIONED.

ELECTRICAL SERVICE FOR ILLUMINATED SIGNS

THE PAY ITEMS IN THE LIGHTING GENERAL SUMMARY INCLUDE THE PULL BOX ADJACENT TO EACH LIGHTED SIGN AND THE ELECTRICAL SERVICE CONNECTIONS LEADING INTO THE BOX, INCLUDING SPLICES OR CONNECTOR KITS IN THE PULL BOX. QUANTITIES FOR ELECTRICAL SERVICE FROM THE CONNECTION IN THE PULL BOX TO THE SIGN ARE INCLUDED IN THE TRAFFIC CONTROL GENERAL SUMMARY.

ALL SIGN SERVICE TAPS TO LIGHTING CIRCUITS WILL BE MADE WITH SPLICE KITS PER 713.15, 5. SIGN FEED TO DISCONNECT SWITCH SHALL BE 5000 VOLT CABLE.

PADLOCKS AND KEYS

PADLOCKS FURNISHED SHALL BE EITHER BRASS OR BRONZE, EQUAL TO MASTER NO. 4BKA OR WILSON BOHANNAN 660A, AND SHALL BE KEYED IN ACCORDANCE WITH SPECIFICATION 631.08, PARAGRAPH 3. PAYMENT SHALL BE INCLUDED IN THE BID FOR THE ITEM BEING LOCKED.

713.07 POLYVINYL CHLORIDE PLASTIC CONDUIT

CONDUIT FURNISHED UNDER THIS SPECIFICATION SHALL CONFORM TO NEMA STANDARDS PUBLICATION NO. TC–6 WITH THE EXCEPTION THAT CONDUIT AND CONDUIT FITTINGS COMPOSED OF ACRYLONITRILE– BUTADIENE–STYRENE (ABS) SHALL NOT BE ACCEPTABLE.

AS AN ALTERNATE TO POLYVINYL CHLORIDE, CORRUGATED COILABLE POLYPROPYLENE CONFORMING TO NEMA STANDARDS PUBLICATION NO. TC–5 MAY BE USED.

ITEM 625 – POWER SERVICE, AS PER PLAN

POWER SERVICE EQUIPMENT SHALL BE INSTALLED AS DETAILED ON SHEET NO. 392. PAYMENT WILL BE MADE FOR EACH "240/480 3 WIRE GROUNDED SERVICE" ITEM 625 – POWER SERVICE, AS PER PLAN" SO INSTALLED AND SHALL INCLUDE ALL LABOR, MATERIALS AND EQUIPMENT NEEDED TO COMPLETE THE INSTALLATION INCLUDING ANY FEES CHARGED BY THE UTILITY COMPANY.

LOW AND HIGH MAST LUMINAIRES

THE LUMINAIRE ARRAYS AND ASSOCIATED ILLUMINATION TEST AREAS SPECIFIED IN SECTION 713.21 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS ARE HEREBY WAIVED FOR THIS PROJECT. INSTEAD, THE LUMINAIRES FOR TOWER LIGHTING SHALL MEET THE FOLLOWING REQUIREMENTS.

SYMMETRIC, TYPE V, LUMINAIRES FOR TOWER LIGHTING MAY BE HOLOPHANE "HMST" TEST #36383, OR GENERAL ELECTRIC "HM" TEST #6312, OR COOPER "HMX" TEST #48381.

IN ADDITION, OTHER LUMINAIRES WILL BE CONSIDERED IF THE DESIGNED INTENSITY AND UNIFORMITY ARE PROVIDED USING THE DESIGNED POLE LOCATIONS AND THE DESIGNED NUMBER AND TYPE OF FIXTURES PER POLE.

LOW MAST LIGHT POLE, DESIGN AT II BB 51.7, AS PER PLAN

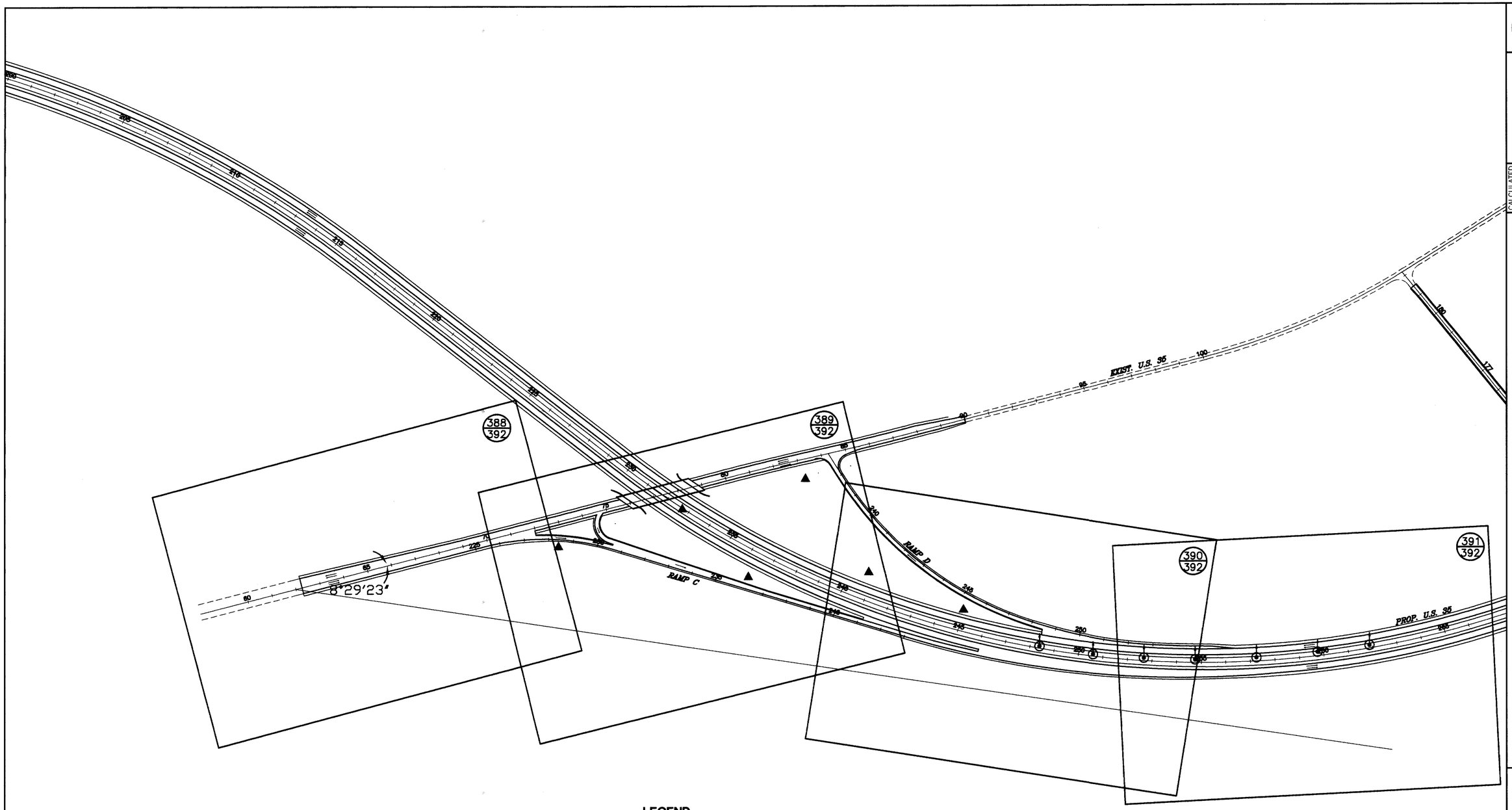
LOW MAST LIGHT POLES WILL INCLUDE 2 SYMMETRIC, TYPE V, 400 WATT HIGH PRESSURE SODIUM LUMINAIRES.

CONDUIT, 3" 713.04, AS PER PLAN

ALL 3" CONDUIT SHALL BE LOCATED A MINIMUM OF 36" DEEP IN ORDER TO CLEAR THE PROPOSED SHALLOW UNDERDRAINS ON MAINLINE AND RAMP C.

FAY-35-2.57

Q:\3590\LIGHTING\3590LTN



LEGEND

- ▲ LIGHT TOWER WITH SYMMETRIC (TYPE V), 400 WATT HIGH PRESSURE SODIUM LUMINAIRES
- LOW MAST LIGHT POLE WITH 2 SYMMETRIC (TYPE V), 400 WATT HIGH PRESSURE SODIUM LUMINAIRES
- (A–1) POLE NUMBER
- PULLBOX OR JUNCTION BOX LIGHTING
- LIGHTING CONTROL CENTER
- DUCT CABLE LOCATION
- PROPOSED 3" DIA. CONDUIT FOR CROSSUNDERS
- HPS HIGH PRESSURE SODIUM
- DRAINAGE STRUCTURE
- ① STUBOUT FOR FUTURE CONNECTIONS
- ② 3" CROSSUNDER CONDUIT TO BE CONTINUOUS UP THRU CONCRETE FOUNDATION USING 3" STEEL CONDUIT ELBOWS
- EXISTING LIGHT POLE
- EXISTING PULLBOX
- OVERHEAD SIGN
- GROUND MOUNTED SIGN

HORIZONTAL SCALE IN FEET
0 100 200 400

CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96

LIGHTING SCHEMATIC PLAN

FAY-35-2.57

2 \ 11

385
522

Q:\3590\LIGNTING\3590LSCH

SHEET NOS.	SUB-SUMMARY SHEET NO. 387				ITEM NO.	ITEM EXT.	GRAND TOTAL	UNIT	DESCRIPTION	LINE NO.
384	388	389	390	391						1
										2
140					603	00400	140	LIN FT	4" CONDUIT, TYPE E	3
										4
										5
										6
			6	8	625	00500	14	EACH	CONNECTOR KIT, TYPE II	7
										8
										9
	6	12	3		625	01500	21	EACH	CABLE SPLICING KIT	10
										11
										12
										13
			3	4	625	10471	7	EACH	LOW MAST LIGHT POLE, DESIGN ATON 51.7, AS PER PLAN (SEE SHEET 384)	14
										15
										16
										17
										18
										19
										20
		1			625	13200	1	EACH	LIGHT TOWER, BBBB 100	21
		1			625	13204	1	EACH	LIGHT TOWER, BBBB 110	22
		2	1		625	13400	3	EACH	LIGHT TOWER, BBBBBB 100	23
										24
		1			625	13406	1	EACH	LIGHT TOWER, BBBBBB 120	25
										26
										27
			3	4	625	14200	7	EACH	LIGHT POLE FOUNDATION, 24" X 10' DEEP	28
										29
		5	1		625	15300	6	EACH	LIGHT TOWER FOUNDATION, 36" X 30' DEEP	30
										31
										32
										33
										34
		1			625	21200	1	EACH	LIGHT TOWER MAINTENANCE PLATFORM, TYPE C	35
										36
										37
	450	1698	282		625	23200	2430	LIN FT	NO. 4 AWG 5000 VOLT DISTRIBUTION CABLE	38
		75			625	23300	75	LIN FT	NO. 2 AWG 5000 VOLT DISTRIBUTION CABLE	39
	430	1486	1233	925	625	24320	4074	LIN FT	1-1/2" DUCT CABLE WITH THREE NO. 4 AWG 5000 VOLT CABLES	40
		617			625	24330	617	LIN FT	1-1/2" DUCT CABLE WITH THREE NO. 2 AWG 5000 VOLT CABLES	41
										42
										43
	143	297	84		625	25500	524	LIN FT	CONDUIT, 3", 713.04	44
		244			625	25501	244	LIN FT	CONDUIT, 3", 713.04, AS PER PLAN (SEE SHEET 384)	45
										46
										47
										48
										49
										50
										51
										52
										53
										54
		26	6		625	26260	32	EACH	LUMINAIRE, HIGH MAST, SYMMETRIC, TYPE V, 400 WATT H.P.S., 713.21, 480 VOLT	55
			6	8	625	26270	14	EACH	LUMINAIRE, LOW MAST, SYMMETRIC, TYPE V, 400 WATT H.P.S., 713.21, 480 VOLT	56
										57
										58
										59
										60
										61
	143	2476	1319	890	625	29002	4828	LIN FT	TRENCH, 24" DEEP	62
										63
										64
										65
	2	4	1		625	30700	7	EACH	PULL BOX, 713.08, 18"	66
										67
		10	5	4	625	32000	19	EACH	GROUND ROD	68
										69
		1			625	34001	1	EACH	POWER SERVICE, AS PER PLAN (SEE SHEET 384)	70
										71
										72
		LUMP			625	38000	LUMP		HIGH VOLTAGE TEST	73
										74
										75
										76
										77
										78
										79
										80
										81
										82

CALCULATED EAH 9/96
CHECKED SAW 9/96

LIGHTING GENERAL SUMMARY

FAY-35-2.57

3 \ 11

386
522

Q:\3590\LIGHTING\3590LTGS

Revised 8/11/00, by E.R.P.

LIGHTING SUB-SUMMARY

CALCULATED EAH 9/96 — CHECKED SAW 9/96

SHEET NO.	REFERENCE NO.	STATION TO STATION	SIDE	ITEM 625																									REMARKS
				CONNECTOR KIT, TYPE II			CABLE SPLICING KIT	LOW MAST LIGHT POLE, DESIGN AT ON , 51.7, AS PER PLAN	LIGHT TOWER, BBBB 100	LIGHT TOWER, BBBB 110	LIGHT TOWER, BBBBBB 100	ON	LIGHT TOWER, BBBBBB 120	LIGHT TOWER FOUNDATION 24" X 10' DEEP	LIGHT TOWER FOUNDATION 36" X 30' DEEP			LIGHT TOWER MAINTENANCE PLATFORM, TYPE C	#2 AWG 5000 VOLT DISTRIBUTION CABLE	#4 AWG 5000 VOLT DISTRIBUTION CABLE	1-1/2" DUCT CABLE W/3 #2 AWG 5000 VOLT CABLE	1-1/2" DUCT CABLE W/3 #4 AWG 5000 VOLT CABLE	CONDUIT, 3" 713.04	CONDUIT, 3" 713.04, AS PER PLAN	LUMINAIRE HIGH MAST SYMMETRIC, TYPE V 400W HPS, 713.21, 480V	LUMINAIRE LOW MAST SYMMETRIC, TYPE V 400W HPS, 713.21, 480V	TRENCH 24" DEEP	PULL BOX 713.08, 18"	GROUND ROD	POWER SERVICE, AS PER PLAN	HIGH VOLTAGE TEST	
				EACH			EACH	EACH	EACH	EACH	EACH		EACH	EACH	EACH			EACH	LIN FT	LIN FT	LIN FT	LIN FT	LIN FT	LIN FT	EACH	EACH	LIN FT	EACH	EACH	EACH	LUMP	
388	PB-5	64+85 OLD 35	RT				3																				1				
388		64+85 OLD 35 TO 66+27.5 "C"	RT																			143				143					
388	PB-4A	66+27.5 OLD 35	RT				3																				1				
388		66+27.5 OLD 35 TO 226+00 "C"	RT																	450		430									
		TOTAL SHEET NO.388		—			6	—	—	—	—		—	—	—			—	—	450	—	430	143	—	—	—	143	2	—	—	—	
389		226+00 "C" TO 226+37	RT																			46				36					
	PB-4	226+37 "C"	RT				3																				1				
		226+37 "C" TO 228+40 "C"	RT																			214				204					
	A-4	228+40 "C"	RT							1					1			1							4			2			
		228+40 "C" TO 236+22.5 "C"	RT																	237		795		69		854	1				
	PB-3A	236+22.5 "C"	RT				3																								
		236+22.5 "C" TO 236+35	R&L																												
	A-3	236+35	RT						1						1										4			2			
		236+35 TO 236+35	RT																	300				90		90					
	A-2	232+70	℄								1				1										6			2			
		232+70 TO 236+35	RT																			379				369					
	PB-3	236+35	RT				3																				1				
		236+35 TO 236+35	R&L																	285				85		85					
	CC-1	83+00 OLD 35	RT																										1	LUMP	
		83+00 OLD 35 TO 83+15 OLD 35	RT																75				15			15					
	PB-1	83+15 OLD 35	RT				3																				1				
		83+15 OLD 35 TO 83+15 OLD 35	RT																		147					39					
	A-1	83+15 OLD 35	RT										1		1										6			2			
		83+15 OLD 35 TO 236+35	RT																	876			282			282					
		83+15 TO 240+75	LT																		470					460					
	A-5	240+75	LT								1				1										6			2			
389		240+75 TO 241+00	LT																			52				42					
		TOTAL SHEET NO. 389		—			12	—	1	1	2	—	1	—	5			1	75	1698	617	1486	297	244	26	—	2476	4	10	1	LUMP	
390		241+00 TO 245+00	LT																			398				435					
	A-6	245+00	LT								1				1										6			2			
		245+00 TO 245+00	LT																	282			84			84					
	PB-7	245+00	LT				3																				1				
		245+00 TO 248+40	LT																			350				340					
	A-7	248+40	LT	2				1						1											2			1			
		248+40 TO 250+60	LT																			230				220					
	A-8	250+60	LT	2				1						1											2			1			
		250+60 TO 252+70	LT																			220				210					
	A-9	252+70	LT	2				1						1											2			1			
390		252+70 TO 253+00	LT																			35				30					
		TOTAL SHEET NO. 390		6			3	3	—	—	1		—	3	1			—	—	282	—	1233	84	—	6	6	1319	1	5	—	—	
391		253+00 TO 254+75	LT																			180				175					
	A-10	254+75	LT	2				1						1											2			1			
		254+75 TO 257+25	LT																			260				250					
	A-11	257+25	LT	2				1						1											2			1			
		257+25 TO 259+75	LT																			260				250					
	A-12	259+75	LT	2				1						1											2			1			
		259+75 TO 261+90	LT																			225				215					
391	A-13	261+90	LT	2				1						1											2			1			
		TOTAL SHEET NO. 391		8			—	4	—	—	—		—	4	—			—	—	—	—	925	—	—	—	8	890	—	4	—	—	

FAY-35-2.57

Q:\3590\LIGHTING\3590LTSS

CONCRETE
TANK
WALL
EX. R/W
PROP. R/W
+25
12'
15'
EXIST. U.S.R. 35
60
12'
12'
12'
65
10
SERVICE RD. NO. 2
9
143'~3" CONDUIT
10' SHLD.
+85.47
24'
24'
70
MATCH LINE STA. 226+00
SEE SHEET NO. 389
EX. R/W
PROP. R/W
PROP. SIGN, NO. 23, 64+75
350 WATT LIGHT
PB-5 64+85, 42.5' RT.
18" CONC.
PB-4A
66+27.5, 42.5' RT.
11
SERVICE RD. NO. 1
PROP. R/W
+02.98
PROP. R/W
RES. DR.
12
13
P.C. STA. C225+40.19 =
24.00' LT., STA. 69+87.48
EX. U.S.R. 35
2 STY FR. RES.
TANK
1 STY BLOCK GARAGE

FOR LEGEND, SEE SHEET NO. 385

FOR QUANTITIES, SEE SHEET NO. 387

FOR DETAILS, SEE SHEET NO. 392

Q:\3590\LIGHTING\3590LT4A

N

0 25 50 100
HORIZONTAL SCALE IN FEET

CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96

LIGHTING PLAN
STA. 60+00 TO STA. 71+00 - EXIST. U.S. 35

FAY-35-2.57

5 \ 11

388
522

SEE SHEET NO. 388
MATCH LINE STA. 226+00

LA R/W

GR

71 72 73 74 75 76 77 78 79 80 81 EX. U.S. 35 82 83 84 85

SERVICE RD. 3 17 18 19 20 21

227 228 229 230 231 232 RAMP "C" 233 234 235 236 237 PROP. U.S. 35 238 239 RAMP "D" 240

1188

UGT

10' SHLD.

6' SHLD.

12' SHLD.

24'

C.B.

E.P.

EX. OH. POWER

226+27 RAMP "C" PROP. SIGN NO. 29 350 WATT LIGHT

226+37 RAMP "C" PB-4, 18.5' RT.

A-4 STA. 228+40, 25' RT. RAMP "C"
LIGHT TOWER, BBBB 4 SYM.
110', 713.21
MAINTENANCE PLATFORM TYPE C

A-2 STA. 232+70, ℄
LIGHT TOWER, BBBBBB, 6 SYM.
100', 713.21

CC-1 STA. 83+00, 36' RT.

PB-1 STA. 83+15, 36' RT.

A-1 STA. 83+15, 75' RT.
LIGHT TOWER, BBBBBB, 6 SYM.
120', 713.21

SEE DETAIL

A-5 STA. 240+75, 115' LT.
LIGHT TOWER, BBBBBB
6 SYM. 100', 713.21

PB-3 STA. 236+35, 10' RT.

90'~3" CONDUIT

69'~3" CONDUIT

PB-3A 236+22.5

A-3 STA. 236+35, 100' RT.
LIGHT TOWER, BBBB 4 SYM. 100', 713.21

MATCH LINE STA. 241+00
SEE SHEET NO. 390

NOTE : DURING THE FINAL PROCESSING OF THESE PLANS, THE PROPOSED PAVED SHOULDER WIDTHS ON U.S. 35 WERE REVISED. THE PAVED SHOULDER WIDTHS WERE REDUCED FROM 12 FT. (BOTH SIDES) TO 10 FT. (OUTSIDE) AND 6 FT. (INSIDE). THE LIGHTING PLAN SHEETS WERE NOT REVISED.

DETAIL

15'
CC-1
PB-1
15'~3" CONDUIT
A-1

FOR LEGEND, SEE SHEET NO. 385
FOR QUANTITIES, SEE SHEET NO. 387
FOR DETAILS, SEE SHEET NO. 392

HORIZONTAL SCALE IN FEET
0 25 50 100

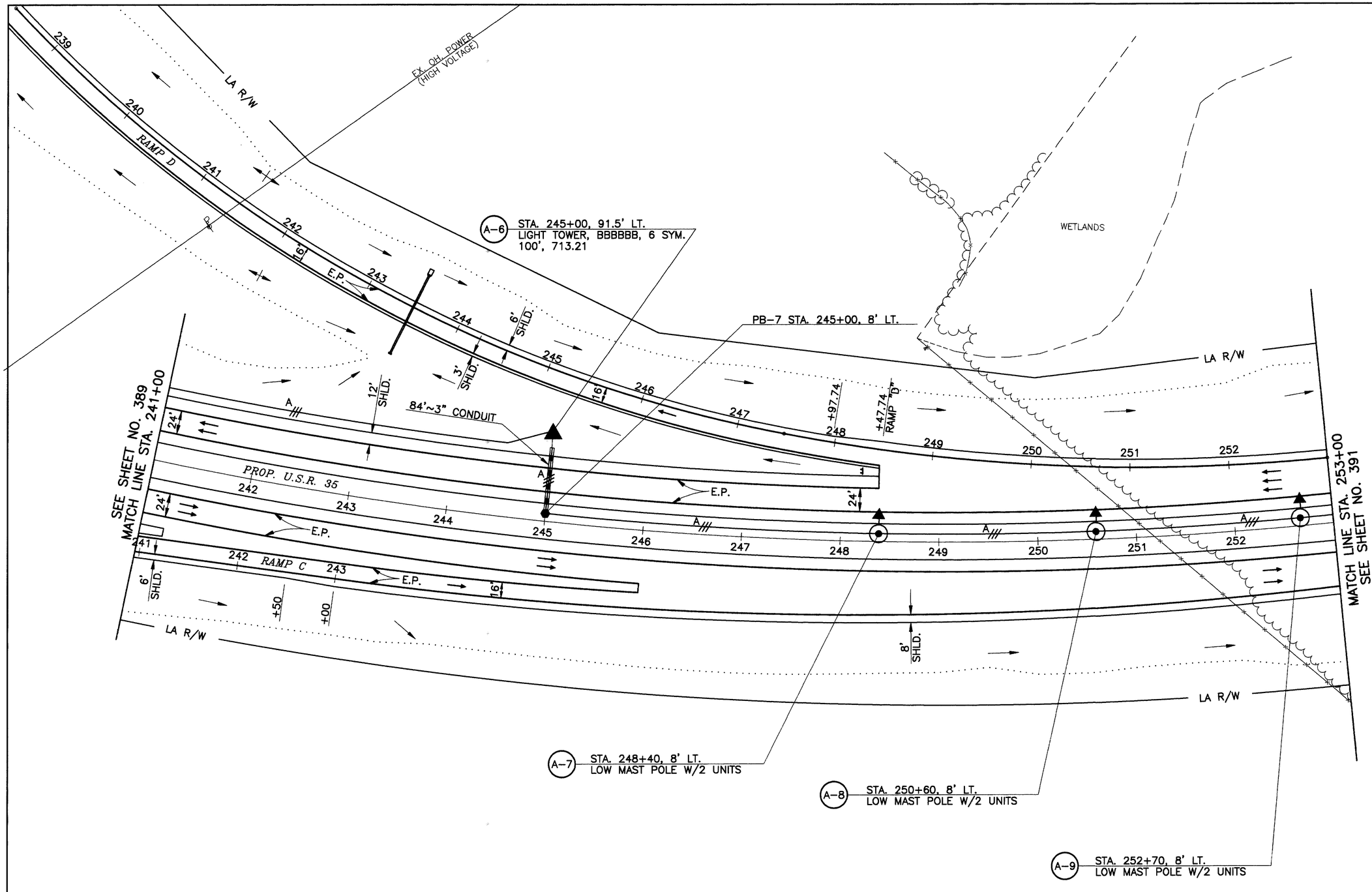
CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96
REVISED 10-99 BY FYC

LIGHTING PLAN
STA. 230+00 TO STA. 241+00

FAY-35-2.57

6 11
389
522

Q:\3590\LIGHTING\3590LT1A



FOR LEGEND, SEE SHEET NO. 385

FOR QUANTITIES, SEE SHEET NO. 387

FOR DETAILS, SEE SHEET NO. 392

CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96

HORIZONTAL SCALE IN FEET
0 25 50 100

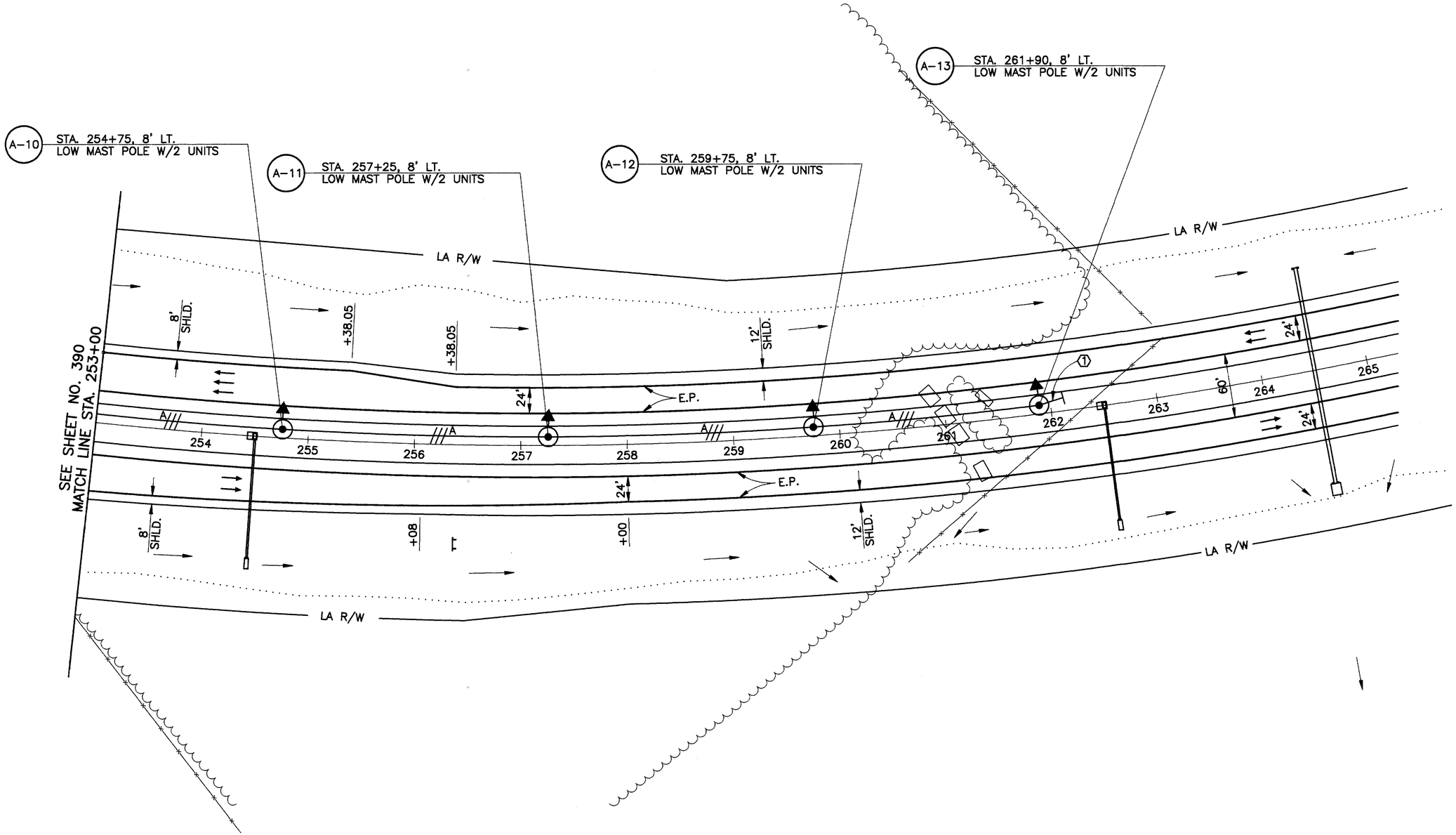
LIGHTING PLAN
STA. 241+00 TO STA. 253+00

FAY-35-2.57

7 \ 11

390
522

Q:\3590\LIGHTING\3590LT2A



FOR LEGEND, SEE SHEET NO. 385

FOR QUANTITIES, SEE SHEET NO. 387

FOR DETAILS, SEE SHEET NO. 392

CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96

0 25 50 100
HORIZONTAL SCALE IN FEET

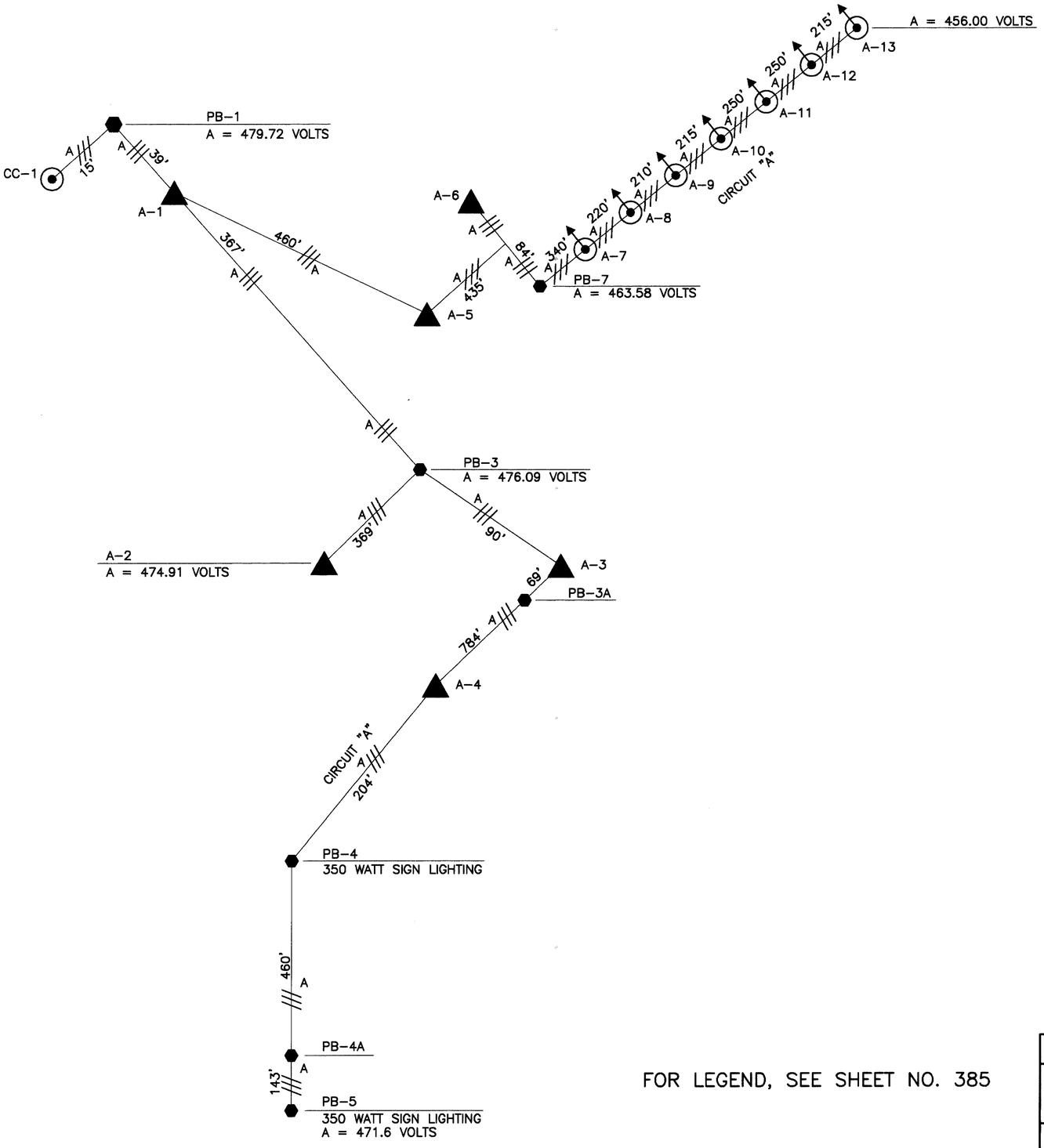
LIGHTING PLAN
STA. 253+00 TO STA. 265+00

FAY-35-2.57

8 \ 11

391
522

Q:\3590\LIGHTING\3590LT3A



LIGHTING PLAN WIRING DIAGRAM

NO SCALE

CIRCUIT "A"

2	@	4 ~	400 WATT LIGHTS	@ 1.1 AMP	=	8.8 AMPS
4	@	6 ~	400 WATT LIGHTS	@ 1.1 AMP	=	26.4 AMPS
7	@	2 ~	400 WATT LIGHTS	@ 1.1 AMP	=	15.4 AMPS
2	@		350 WATT SIGNS	@ 0.9 AMP	=	1.8 AMPS
						52.4 AMPS

FOR LEGEND, SEE SHEET NO. 385

POWER SERVICE DATA

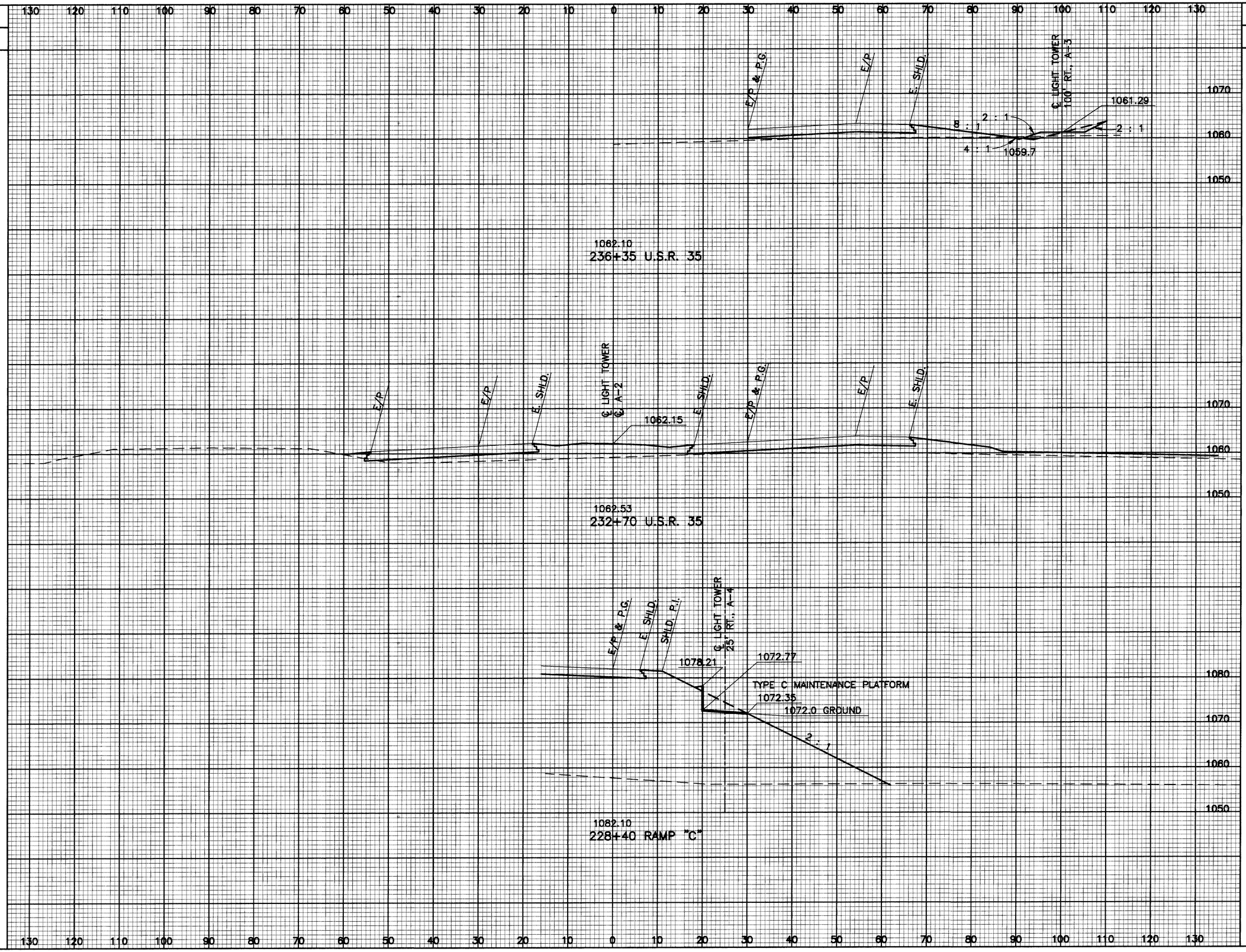
IDENTITY	POWER SERVICE TYPE OF SERVICE	CONNECTED LOAD (KVA)	ENCLOSURE RATING (AMPS)	SERVICE ENTRANCE CABLE SIZE (AWG)	CIRCUIT NUMBER	CIRCUIT LOAD (AMPS)	FUSE SIZE (AMPS)	MAINTAINING AGENCY
CC-1	240/480VOLT 3 WIRE GROUNDED NEUTRAL	26	100	NO. 2	A	53	100	ODOT

CALCULATED E.A.H. 10/96
CHECKED S.A.W. 10/96

LIGHTING DETAILS

FAY-35-2.57

Q:\3590\LIGHTING\3590LTDT



CALCULATED

CHECKED

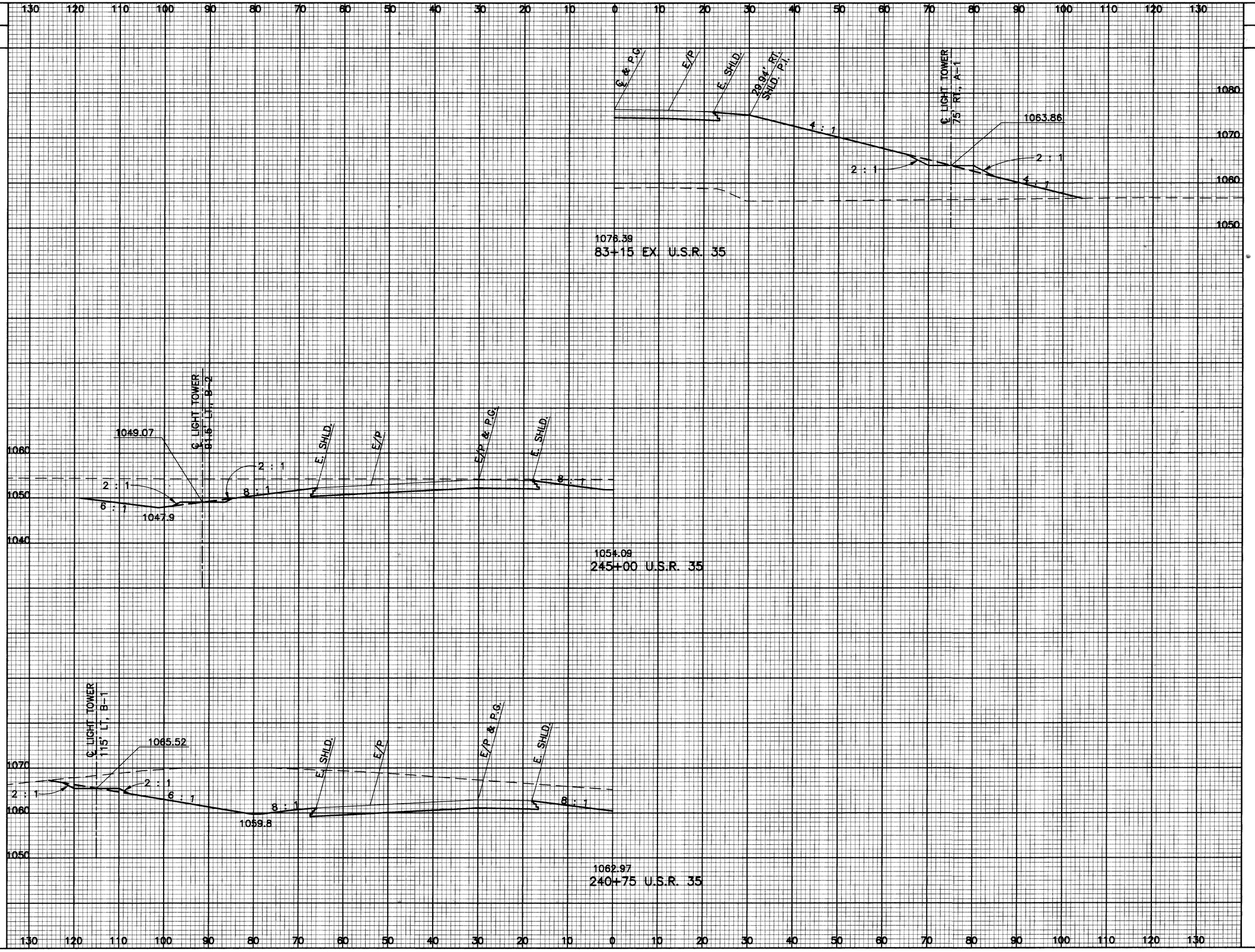
LIGHTING TOWER SECTIONS

FAY-35-2.57

10 \ 11

392A
522

Q:\3590\LIGHTING\90LTXS1



CALCULATED

CHECKED

LIGHT TOWER SECTIONS

FAY-35-2.57

11 \ 11

392B
522

Q:\3590\LIGHTING\90LTXS2

B.M.–3025

Iron Pin With Cap
Sta. 137+90.80, 61.40' Rt.
Elev. = 1048.15

B.M.–3023

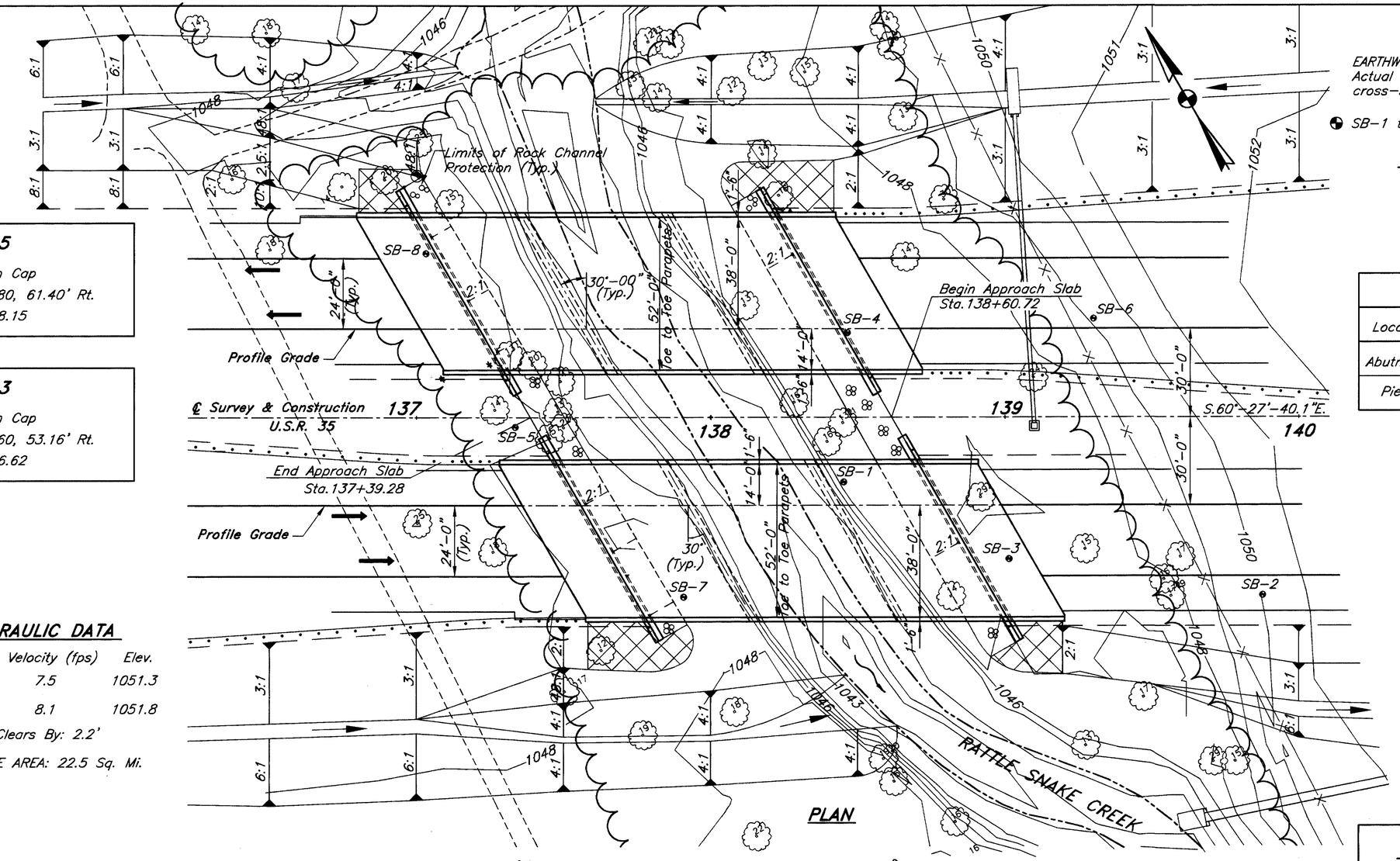
Iron Pin With Cap
Sta. 139+22.60, 53.16' Rt.
Elev. = 1046.62

HYDRAULIC DATA

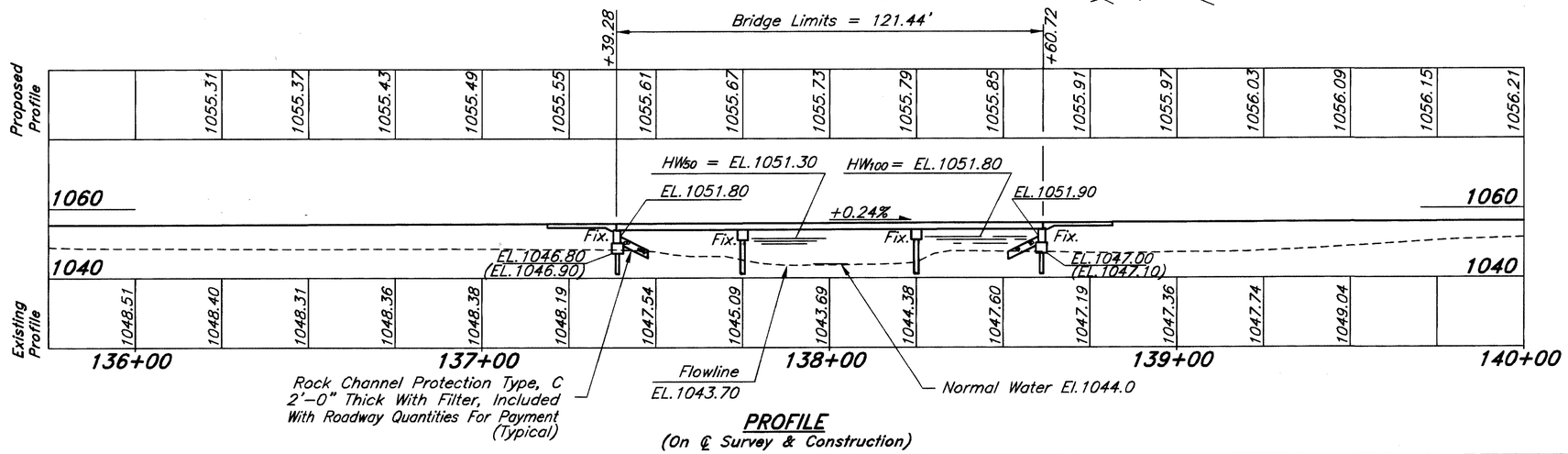
	Flow (cfs)	Velocity (fps)	Elev.
Q_{50} =	2694	7.5	1051.3
Q_{100} =	3085	8.1	1051.8

Q_{50} Clears By: 2.2'

DRAINAGE AREA: 22.5 Sq. Mi.



PLAN



PROFILE
(On ℄ Survey & Construction)

EARTHWORK LIMITS shown are approximate. Actual slopes shall conform to plan cross–sections.

● *SB–1 thru SB–8 indicate Boring Locations.*

DESIGN TRAFFIC

1997 ADT 1420
2017 ADT 1990
18% Trucks

PILE DATA

Location	Type	Estimated Pile Length
Abutments	12"ø Conc.	30'
Piers	16"ø Conc.	50' @ Pier 1 40' @ Pier 2

PROPOSED STRUCTURE

TYPE: Continuous Slab Bridge on Capped Pile Abutments & Piers

SPAN: 35', 50' & 35' c/c Bearings

ROADWAY: 52'–0" Toe to Toe Parapets

LOADING: HS20–44 And The Alternate Military Loading

SKEW: 30°–0" R.F.

WEARING SURFACE: Monolithic Concrete

APPROACH SLABS: 20' Long (Special)

ALIGNMENT: Tangent

CROWN : 0.0156

LATITUDE : 39°–37'–55.65"N

LONGITUDE : 83°–37'–00"W

USGS QUADRANGLE : JEFFERSONVILLE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2–24–99
REVIEWED CEE
DRAWN RT
DESIGNED RT
CHECKED VK
REVISED
STRUCTURE FILE NO. 2400383

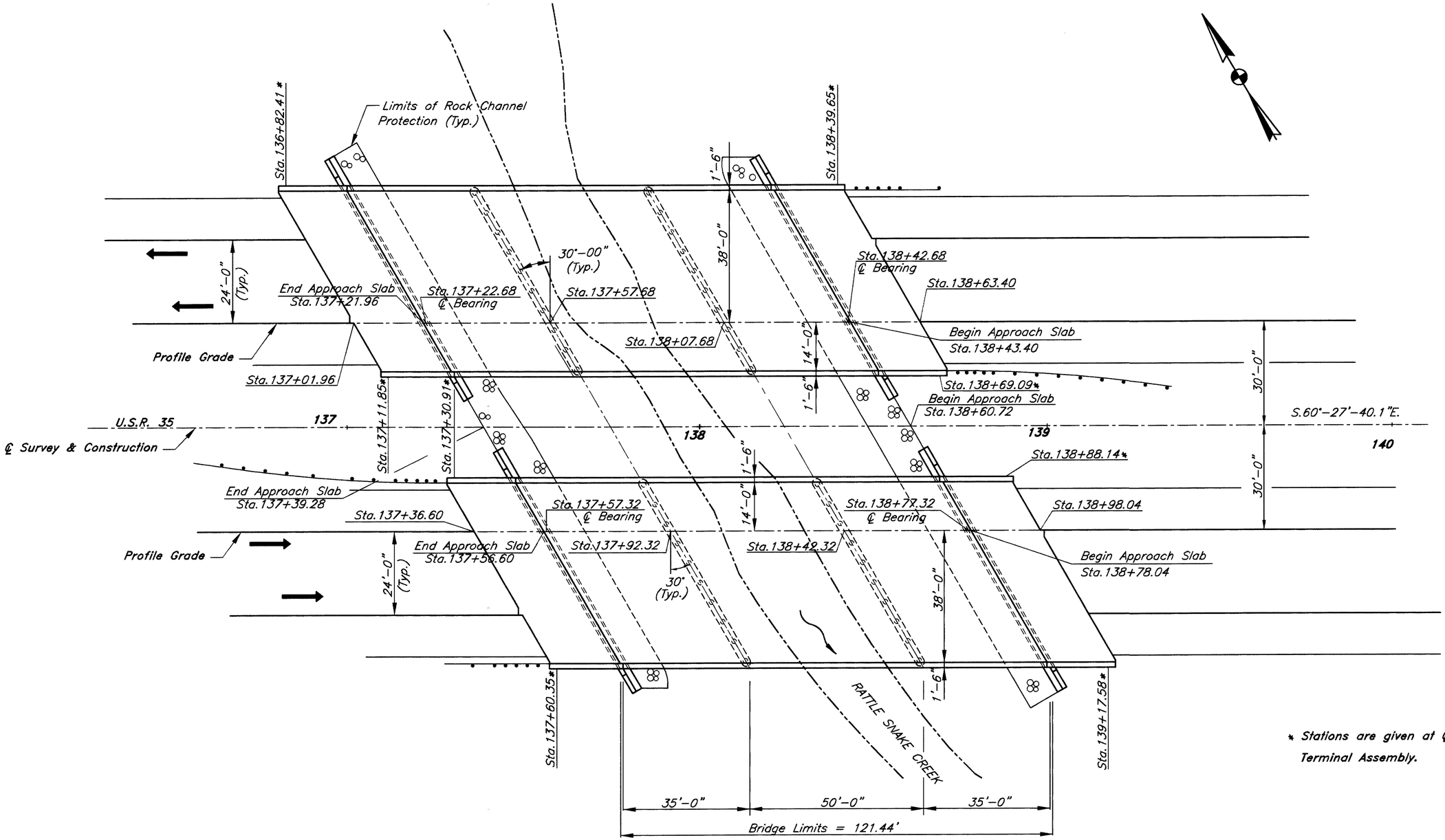
FAYETTE COUNTY
STA. 137+39.28
STA. 138+60.72

SITE PLAN
BRIDGE NO. FAY–35–0258 L & R.
OVER RATTLE SNAKE CREEK

FAY–35–2.57

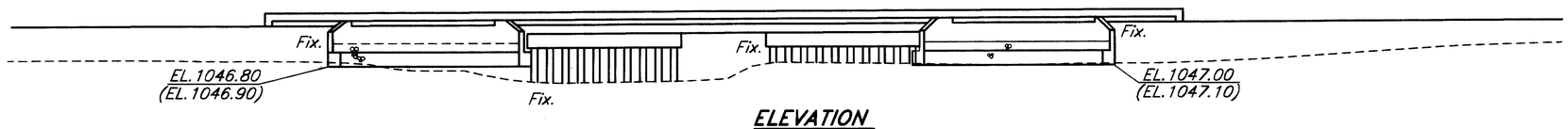
1 / 11

393
522



* Stations are given at ℄ Guardrail Terminal Assembly.

GENERAL PLAN



ELEVATION

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO.:2400383	
RT/VK			

GENERAL PLAN & ELEVATION
BRIDGE NO. FAY-35-0258 L.& R.
OVER RATTLE SNAKE CREEK

FAY-35-2.57

2 / 11

394
522

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844 -	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C -	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL -	ASTM A615, A616 OR A617 GRADE 60 UNIT STRESS 24,000 PSI

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81 REV. 9-15-94 SHEET NO. 1. AND 2.

CPP-2- 94 DATED 12-19-94

CS-1-93 DATED 6-30-95 SHEET NO. 1, 2 & 3

BR-1 DATED 1-06-99

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS:

842 DATED 1-6-99
844 DATED 1-6-99

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, 2 1/2" CONCRETE COVER TOP, 1 1/2" COVER BOTTOM AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 54 TON PER PILE FOR THE 12" 0 CIP ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 120 TONS PER PILE FOR THE 16" CIP PILES.

ABUTMENT PILES:
- 44 PILES 30 FEET LONG, ESTIMATED LENGTH
- 44 PILES OF ORDER LENGTH 35 FEET LONG
- 22 SPLICES

PIER PILES:
- 40 PILES 40, 50 FEET LONG, ESTIMATED LENGTH
- 20 PILES OF ORDER LENGTH 55 FEET LONG
- 20 PILES OR ORDER LENGTH 45 FEET LONG
- 20 SPLICES

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.06 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

ABUTMENT BACKFILL

ABUTMENT BACKFILL SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

CONCRETE PARAPETS:

AS SOON AS A CONCRETE SAW CAN BE OPERATED WITHOUT DAMAGING THE FRESHLY PLACED CONCRETE, 1 INCH DEEP CONTROL JOINTS SHALL BE SAWED INTO THE PERIMETER OF THE CONCRETE PARAPET. THE SAW CUT SHALL BE MADE IN COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK. THE SAWCUTS SHALL BE PLACED AT A MINIMUM OF 6'-0" AND A MAXIMUM OF 10'-0" CENTERS. THE USE OF AN EDGE GUIDE FENCE, OR JIG IS REQUIRED TO INSURE THAT THE CUT JOINT IS STRAIGHT, TRUE, AND ALIGNED ON ALL FACES OF THE PARAPET. THE JOINT WIDTH SHALL BE THE WIDTH OF THE SAW BLADE, A NOMINAL WIDTH OF 1/4" INCH. THE PERIMETER OF THE DEFLECTION CONTROL JOINT SHALL BE SEALED TO A MINIMUM DEPTH OF 1 INCH WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION, TT-S-00227E TO A MINIMUM DEPTH OF 1 INCH. THE BOTTOM ONE HALF INCH OF BOTH THE INSIDE AND OUTSIDE FACES OF THE PARAPET SHOULD BE LEFT UNSEALED TO ALLOW ANY WATER WHICH MAY ENTER THE JOINT TO ESCAPE..

ALL LABOR, TOOLS, EQUIPMENT, MATERIALS AND INCIDENTALS NECESSARY TO COMPLETE THIS WORK ARE INCLUDED UNDER ITEM 842 CLASS S CONCRETE, SUPERSTRUCTURE.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 5/11 , 7/ 11 , AND 10/ 11.
SEE PROPOSAL NOTE FOR SURFACE PREPARATION REQUIREMENTS, APPLICATION RATES, MATERIAL REQUIREMENTS AND APPLICATION PROCEDURES.

ITEM 503, UNCLASSIFIED EXCAVATION, AS PER PLAN

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1922 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED: RT	REVISED	STRUCTURE FILE NO. 2400383	

GENERAL NOTES
BRIDGE NO. FAY-35-0258
U.S.35 OVER RATTLE SNAKE CREEK

FAY-35-2.57

3 / 11

395
522

May 24, 1999 C:\BRIDGE\Fay98\3598sgn.dwg

ESTIMATED QUANTITIES

					LEFT BRIDGE			RIGHT BRIDGE			
ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	ABUT.	PIER	SUPER.	ABUT.	PIER	SUPER.	GEN.
503	21101	347	CU YD	UNCLASSIFIED EXCAVATION, AS PER PLAN	173.5			173.5			
505	11100	LUMP	LUMP	PILE DRIVING EQUIPMENT MOBILIZATION							LUMP
507	00500	1320	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN	660			660			
507	00700	1980	LIN.FT.	16" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN		1100			880		
507	00550	1540	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED	770			770			
507	00750	2200	LIN.FT.	16" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED		1210			990		
507	50500	42	EACH	STEEL PILE SPLICES	11	10		11	10		
SPECIAL	51267500	545	SQ YD	SEALING OF CONCRETE SURFACES *		6.5	266		6.5	266	
SPECIAL	51267502	66	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY) *	33			33			
518	21200	94	CU YD	POROUS BACKFILL WITH FILTER FABRIC	47			47			
518	40000	320	LIN FT	6" PERFORATED CORRUGATED PLASTIC PIPE,	160			160			
518	40010	114	LIN FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE,	57			57			
				INCLUDING SPECIALS							
523	11100	3	HOURS	DYNAMIC LOAD TEST		1.5			1.5		
842	42500	46	CU YD	CLASS C CONCRETE, PIER CAP		23			23		
842	43500	194	CU YD	CLASS C CONCRETE, ABUTMENT	97			97			
844	49000	992	CU YD	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE (DECK)			496			496	
844	49020	84	CU YD	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE (PARAPET)			42			42	
844	49000		LUMP	HIGH PERFORMANCE CONCRETE TRIAL MIX							
844	49010		LUMP	HIGH PERFORMANCE CONCRETE TESTING							
				Computed by: S.P. Date: 11-18-94							
				Checked by: V.K. Date: 11-25-94							

* SEE PROPOSAL NOTE

DESIGN AGENCY
ENGINEERING ENGINEERING LTD.
1830 CHEAPSIDE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99
CHECKED VK | REVISED
STRUCTURE FILE NO.2400383

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0258
U.S.35 OVER RATTLE SNAKE CREEK

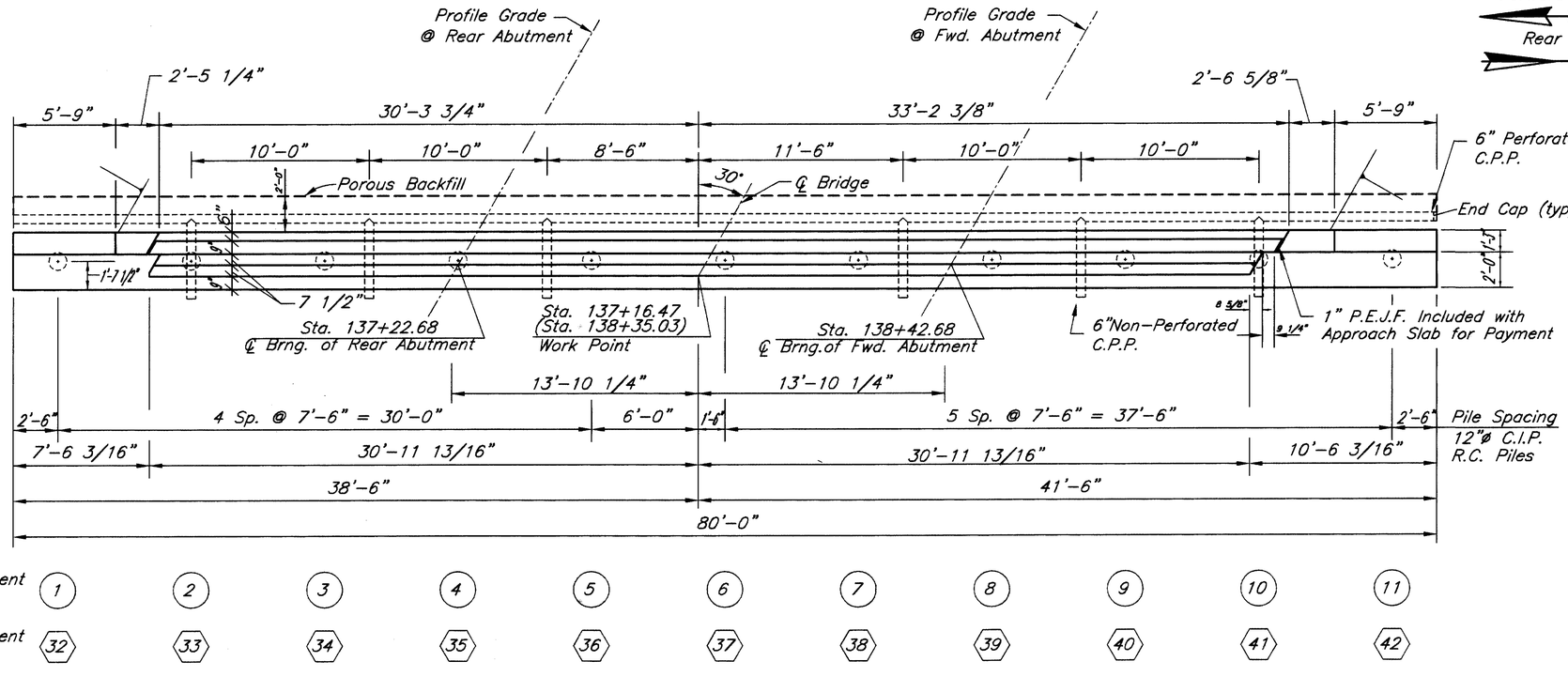
FAY-35-2.57

4 / 11

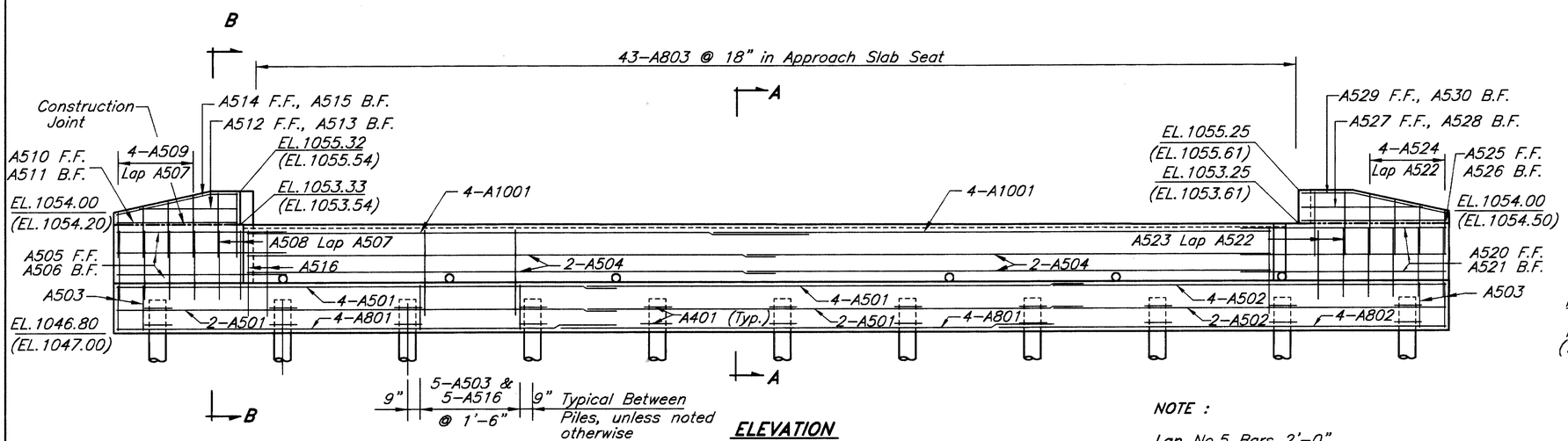
396
522

Revised 8/11/00 by E.R.P.

May 24, 1999 Q:\BRIDGE\Fay35\3586qn.dwg



PLAN–LEFT BRIDGE ABUTMENT



ELEVATION

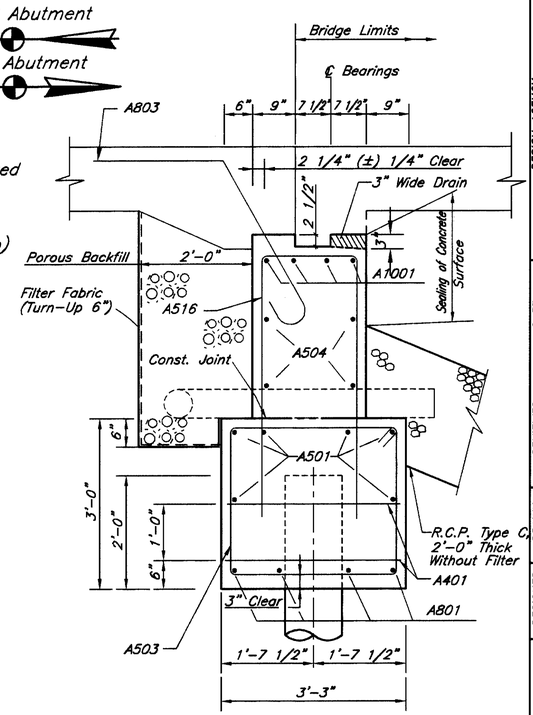
NOTE :

Lap No.5 Bars 2'–0"
No.8 Bars 3'–9"
No.10 Bars 5'–11" minimum.

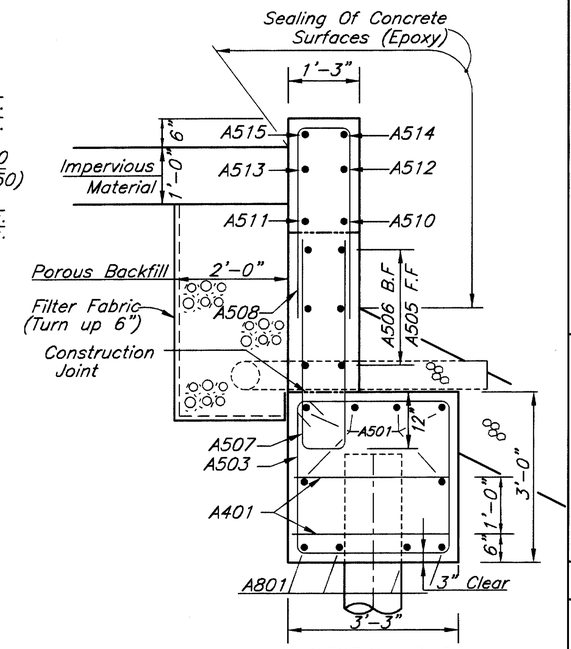
ABBREVIATIONS:

F.F. = Front Face
B.F. = Back Face
C.P.P. = Corrugated Plastic Pipe

Elevations in ( ) are for Forward Abutment



SECTION A–A



SECTION B–B

ABUTMENT DETAILS

BRIDGE NO. FAY-35-0258 (LEFT)
U.S.35 OVER RATTLE SNAKE CREEK

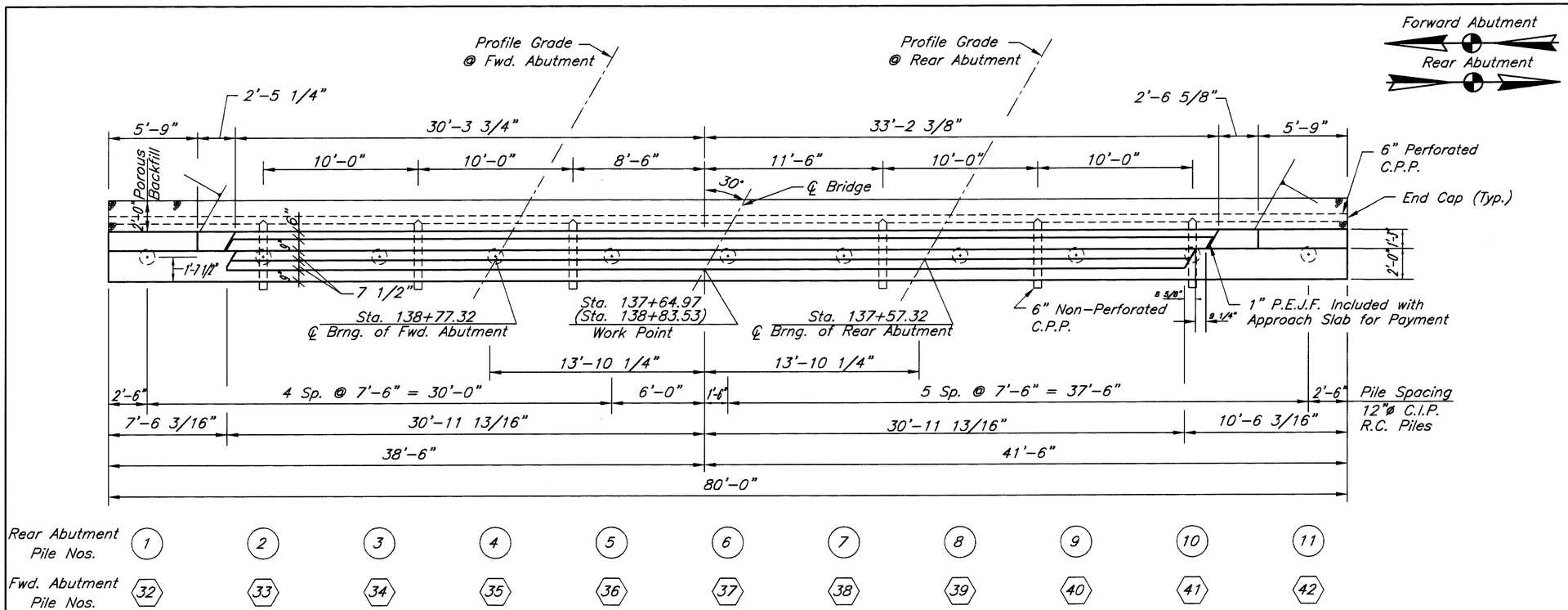
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO. 2400383	
VK			

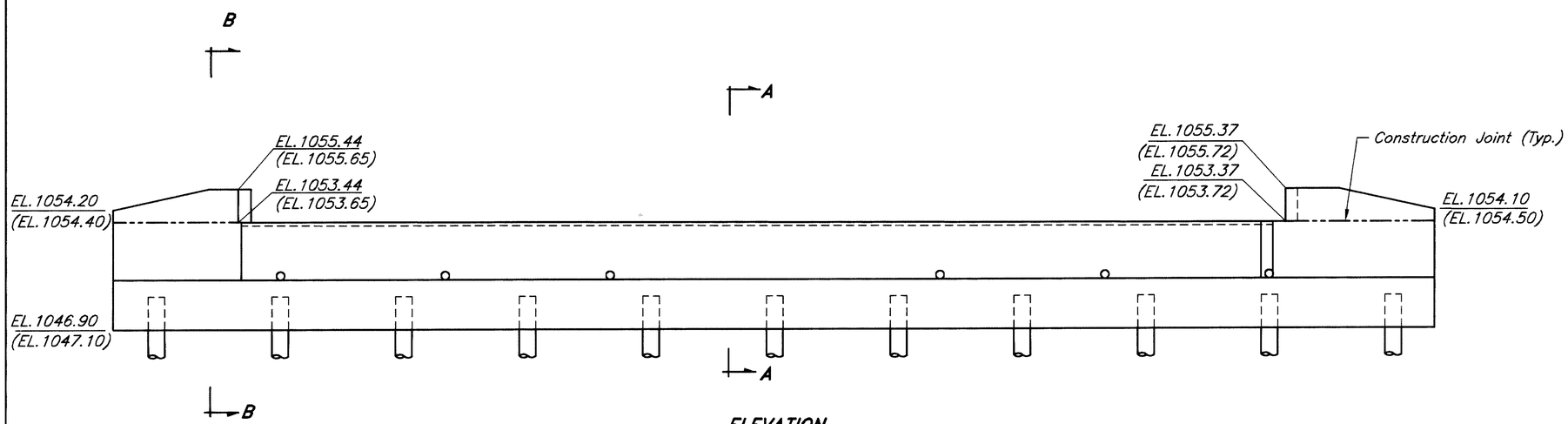
FAY-35-2.57

5 / 11

397 / 522



PLAN–RIGHT BRIDGE



ELEVATION

ABBREVIATIONS:

C.P.P. = Corrugated Plastic Pipe

Elevations in ( ) are for Forward Abutment

For Sections A–A & B–B See Sheet 5/11.

For Reinforcing See Sheet 5/11.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED cee | DATE 2-24-99

STRUCTURE FILE NO. 2400383

REVISED

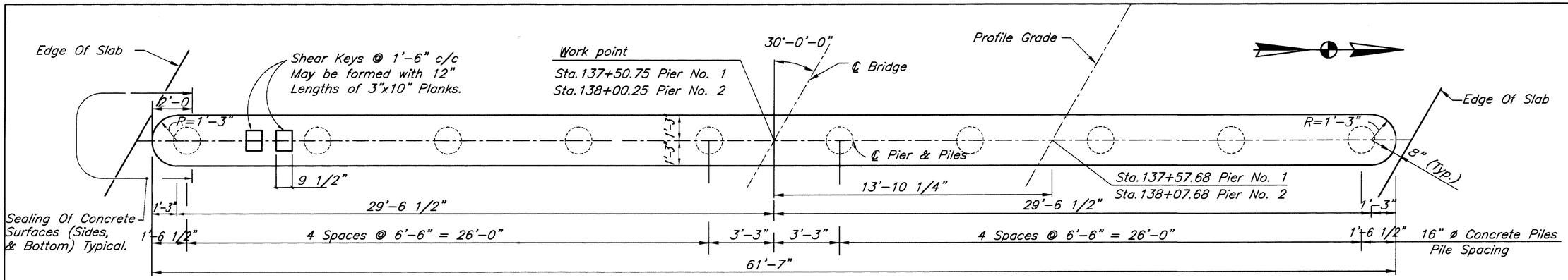
CHECKED VK

ABUTMENT DETAILS
BRIDGE NO. FAY-35-0258 (RIGHT)
U.S.35 OVER RATTLE SNAKE CREEK

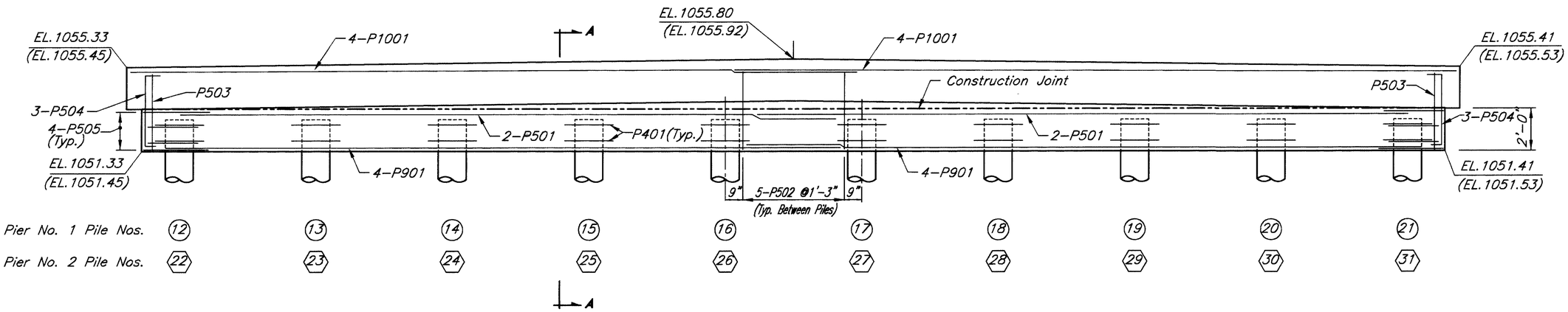
FAY-35-2.57

6 / 11

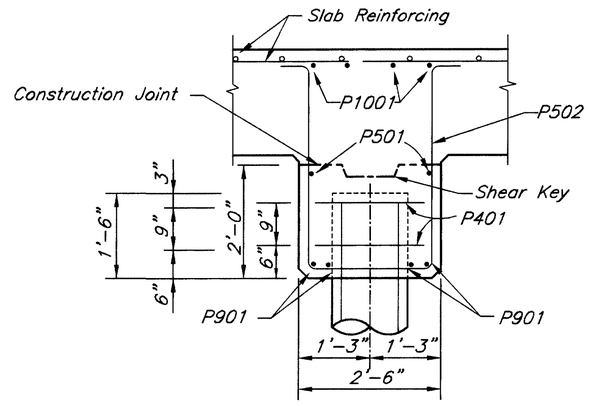
398
522

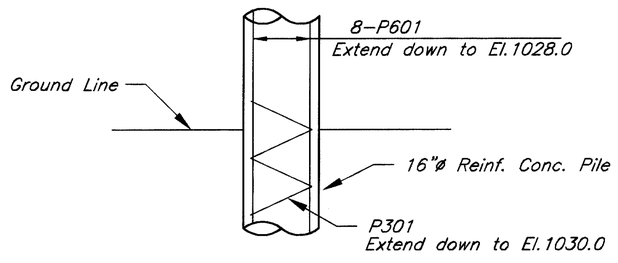


PLAN– LEFT BRIDGE PIERS



ELEVATION





SECTION A–A

NOTE:

* Elevations are given @ ℄ Pier.

Elevations in ( ) are for Pier 2.

LAP No. 5 Bars 2'–0"
No. 9 Bars 4'–8"
No. 10 Bars 5'–11"

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2–24–99
CHECKED	REVISED		
VK			

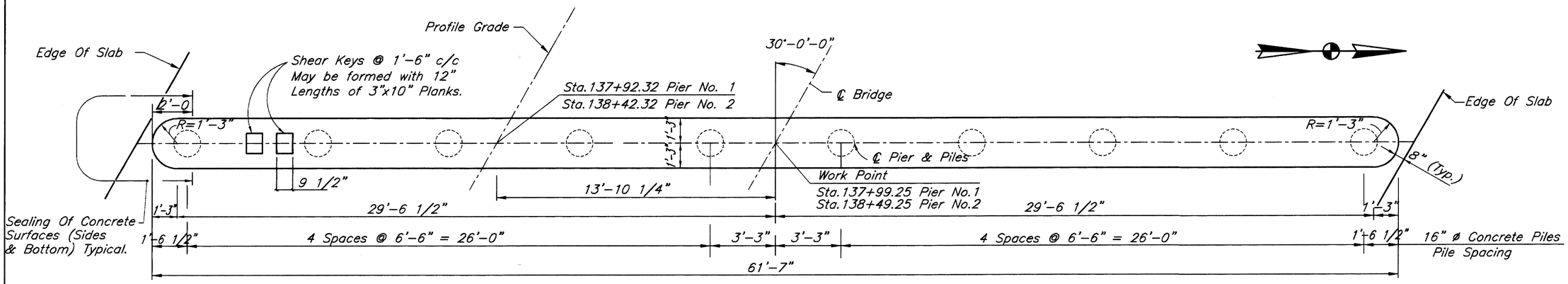
STRUCTURE FILE NO. 2400383

PIER DETAILS
BRIDGE NO. FAY–35–0258 (LEFT)
U.S. OVER RATTLE SNAKE CREEK

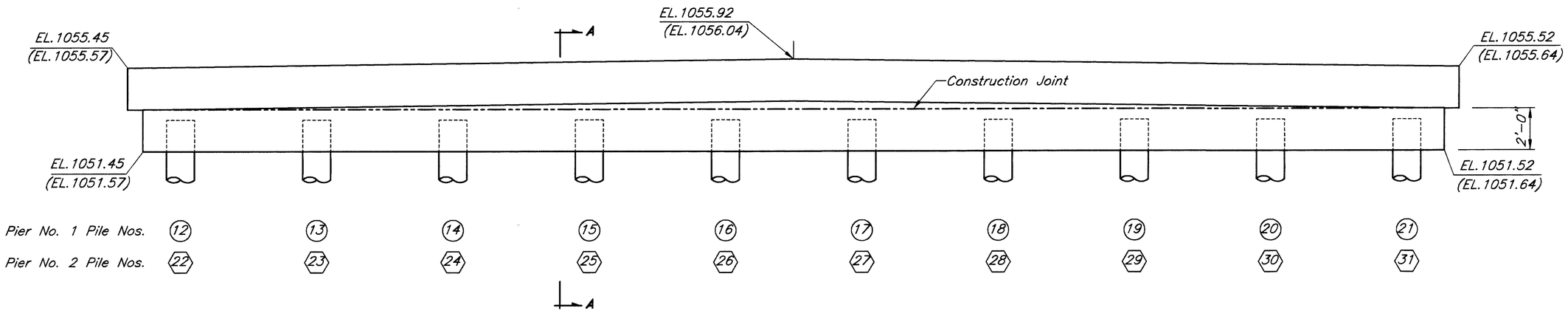
FAY–35–2.57

7 / 11

399
522



PLAN– RIGHT BRIDGE PIERS



ELEVATION

For Reinforcing & Section A–A See Sheet 7 | 11

NOTE:

* Elevations are given @ ℄ Pier.
Elevations in ( ) are for Pier 2.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO. 2400383

DRAWN SP
REVISED

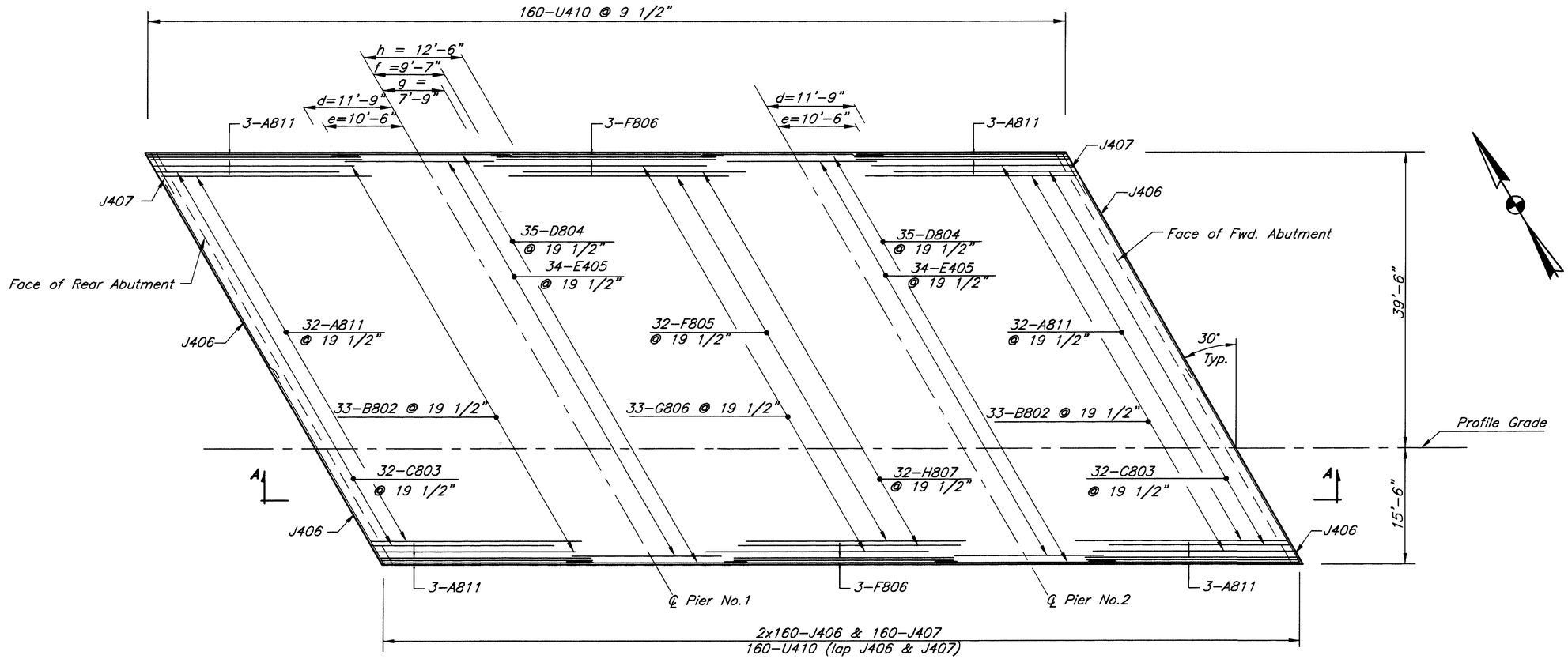
DESIGNED SP
CHECKED VK

PIER DETAILS
BRIDGE NO. FAY-35-0258 (RIGHT)
U.S. OVER RATTLE SNAKE CREEK

FAY-35-2.57

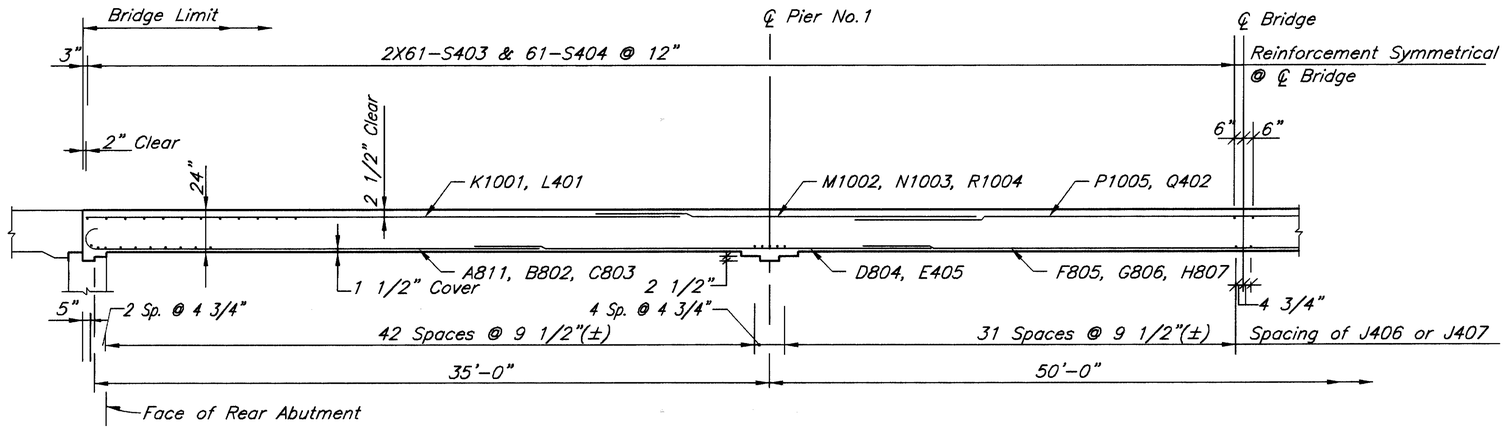
8 | 11

400
522



BOTTOM REINFORCING PLAN–LEFT BRIDGE

Right Bridge Similar



SECTION A–A

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2–24–99
CHECKED	REVISED		
RTI VK			

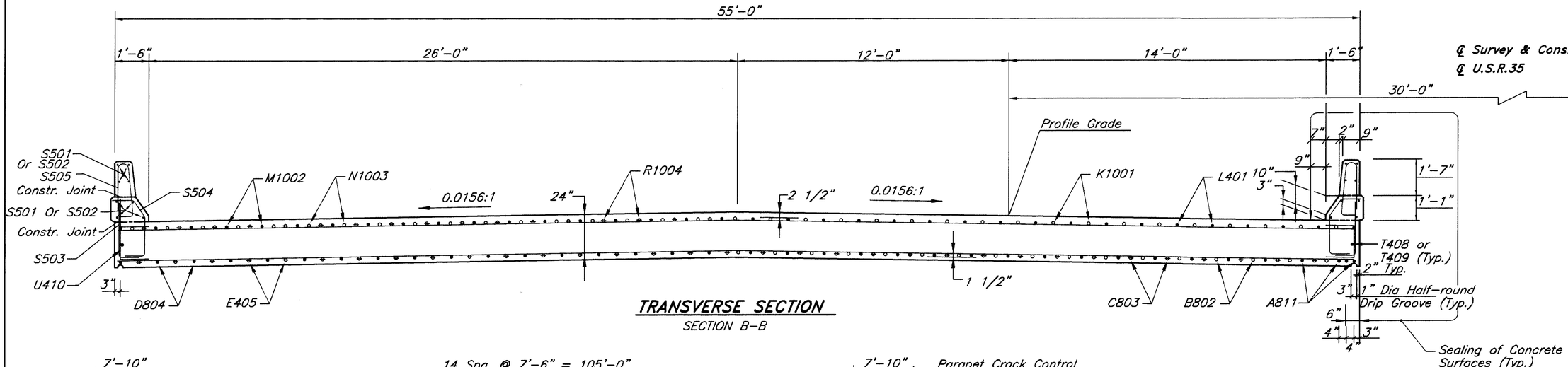
STRUCTURE FILE NO. 2400383

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY–35–0258 L & R.
U.S.35 OVER RATTLE SNAKE CREEK

FAY–35–2.57

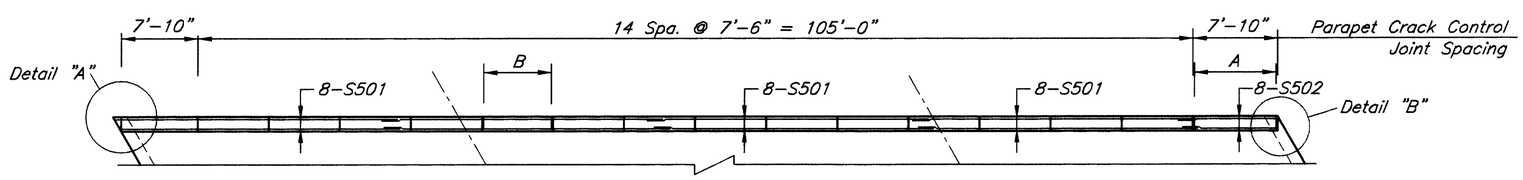
9 / 11

401
522

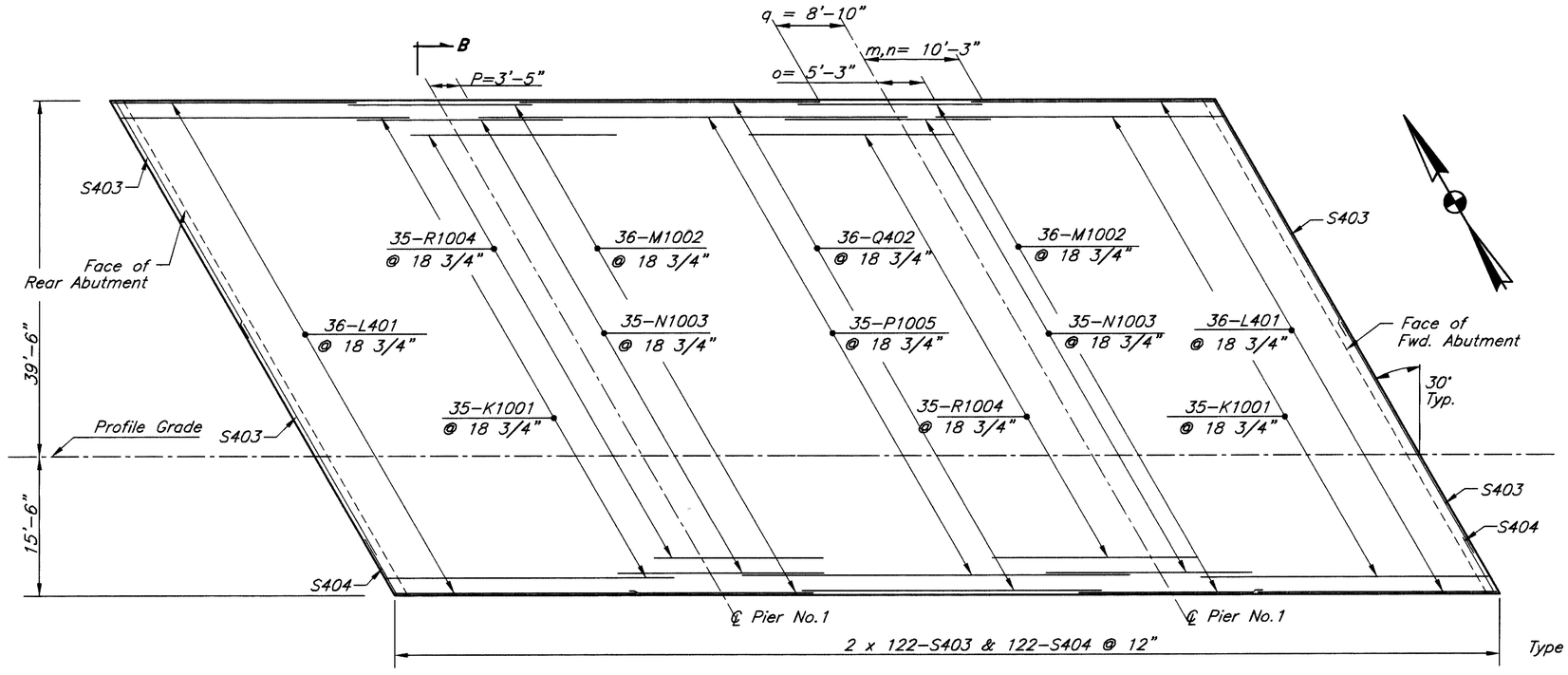


TRANSVERSE SECTION

SECTION B–B



PARAPET REINFORCING DETAIL



TOP REINFORCING PLAN–LEFT BRIDGE

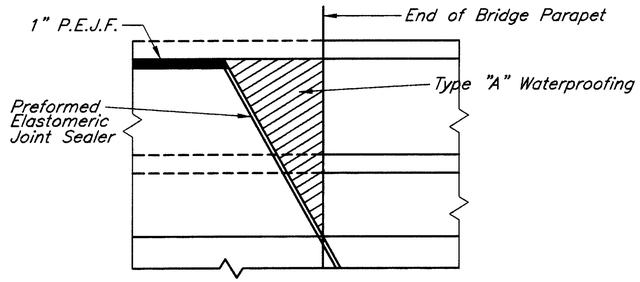
Right Bridge Similar

For Additional Details and Notes Reference is made to Standard Drowing CS–1–93, Sheets No.1,No.2 & No.3

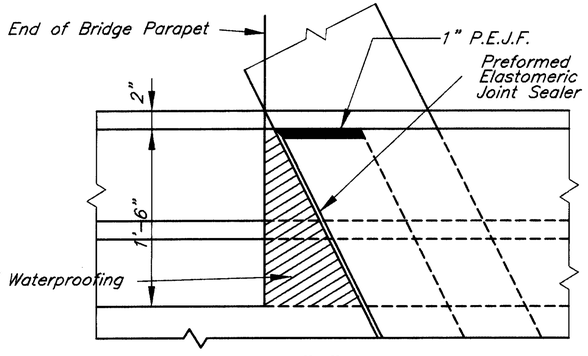
A = 7–S503, S504 & S505 @ 1'–3"

B = 6–S503, S504 & S505 @ 1'–5"

Lap No.4 bars 1'–8" min.
No.5 bars 2'–0" min.
No.8 bars 3'–9" min.
No.10 bars 5'–11" min.



DETAIL "A"



DETAIL "B"

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99
CHECKED VK | REVISED | STRUCTURE FILE NO.: 2400383

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0258 L & R.
U.S.35 OVER RATTLE SNAKE CREEK

FAY-35-2.57

10 | 11

402
522

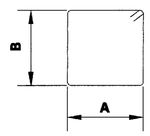
REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
			ABUTMENTS-ONE BRIDGE						
A401	44	9-2	269	1	1-9	2-7 3/4			
A501	24	30-0	751	STR					
A502	12	23-7	295	STR					
A503	108	11-6	1295	1	2-11	2-7			
A504	16	31-9	530	STR					
A505	6	10-2	64	STR					
A506	6	7-8	48	STR					
A507	12	10-0	125	2	0-11	4-8			
A508	4	8-4	35	2	0-11	3-10			
A509	2 sets	6-0 TO	58	2	0-11	2-8 TO			Vary "B" by 4"
	of 4	8-0				3-8			
A510	2	7-3	15	STR					
A511	2	7-10	16	STR					
A512	2	5-10	12	STR					
A513	2	6-5	13	STR					
A514	2	7-3	15	10	1-7	5-8	1-4		
A515	2	7-10	16	10	2-2	5-8	1-4		
A516	84	11-5	1000	2	1-8	5-0			
A520	6	12-0	75	STR					
A521	6	8-0	50	STR					
A522	12	9-0	113	2	0-11	4-2			
A523	4	8-4	35	2	0-11	3-10			
A524	2 sets	6-0 TO	58	2	0-11	2-8 TO			Vary "B" by 4"
	of 4	8-0				3-8			
A525	2	8-6	18	STR					
A526	2	7-11	17	STR					
A527	2	7-0	15	STR					
A528	2	6-6	14	STR					
A529	2	8-5	18	10	2-9	5-8	1-3		
A530	2	8-0	17	10	2-4	5-8	1-3		
A801	16	30-0	1282	STR					
A802	8	27-2	580	STR					
A803	86	5-0	1148	3	2-8	1-5			
A1001	16	33-9	2324	STR					
			10321	LBS X 2 BRIDGES = 20642 LBS					
			PIERS-ONE BRIDGE						
P301	20	21-6	1111	SPIRAL					Pitch 12",
									Core dia.10"
P401	40	8-6	227	1	2-0	2-0			
P501	8	30-0	250	STR					
P502	90	10-0	939	11	2-2	3-5	0-9		
P503	4	9-8	40	11	1-10	3-5	0-9		
P504	12	4-5	55	2	3-5	0-7 1/2			
P505	16	8-2	136	12	3-2	2-6			

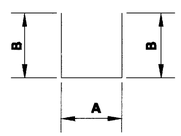
Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
P601	160	23-0	5527	STR					
P901	16	32-0	1741	STR					
P1001	16	34-6	2375	STR					
			12401	LBS X 2 BRIDGES = 24802 LBS					
			SUPERSTRUCTURE-ONE BRIDGE						
L401	72	27-9	1335	STR					
Q402	36	33-7	806	STR					
S403	244	30-0	4890	STR					
S404	122	6-6	530	STR					
E405	68	20-0	908	STR					
J406	320	30-0	6413	STR					
J407	160	6-6	695	STR					
T408	8	30-0	160	STR					
T409	2	7-6	10	STR					
U410	320	3-10	819	2	1-6	1-3			
S501	64	30-0	2003	STR					
S502	16	10-0	167	STR					
S503	196	3-3	664	4	10 1/2	2-6			
S504	196	4-2	852	5	1-10				
S505	196	7-1	1448	6					
A811	76	28-4	5749	STR					
B802	66	26-9	4714	STR					
C803	64	24-11	4258	8	24-0				
D804	70	25-1	4686	STR					
F805	38	30-10	3128	STR					
G806	33	29-9	2621	STR					
H807	32	29-9	2542	STR					
K1001	70	32-0	9639	STR					
M1002	72	20-6	6351	STR					
N1003	70	22-11	6903	STR					
R1004	70	22-11	6903	STR					
P1005	35	43-2	6501	STR					
			85699	LBS X 2 BRIDGES = 171398 LBS					
					PARAPET				
					See sheet 259				

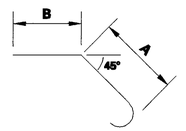
ALL BARS SHALL BE EPOXY COATED



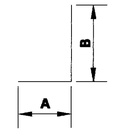
TYPE 1



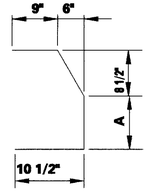
TYPE 2



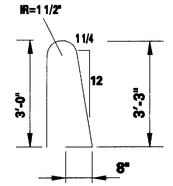
TYPE 3



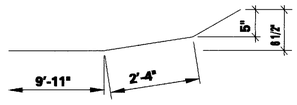
TYPE 4



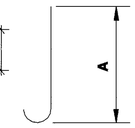
TYPE 5



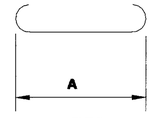
TYPE 6



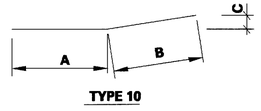
TYPE 7



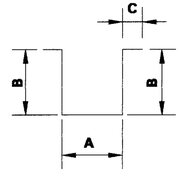
TYPE 8



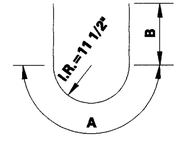
TYPE 9



TYPE 10



TYPE 11



TYPE 12

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Two steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 1.60 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED CEE | DATE 2-24-99 | STRUCTURE FILE NO.2400383

DRAWN SP | REVISED

DESIGNED SP | CHECKED VK

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0258 L. & R.
U.S.35 OVER RATTLE SNAKE CREEK

FAY-35-2.57

11 / 11

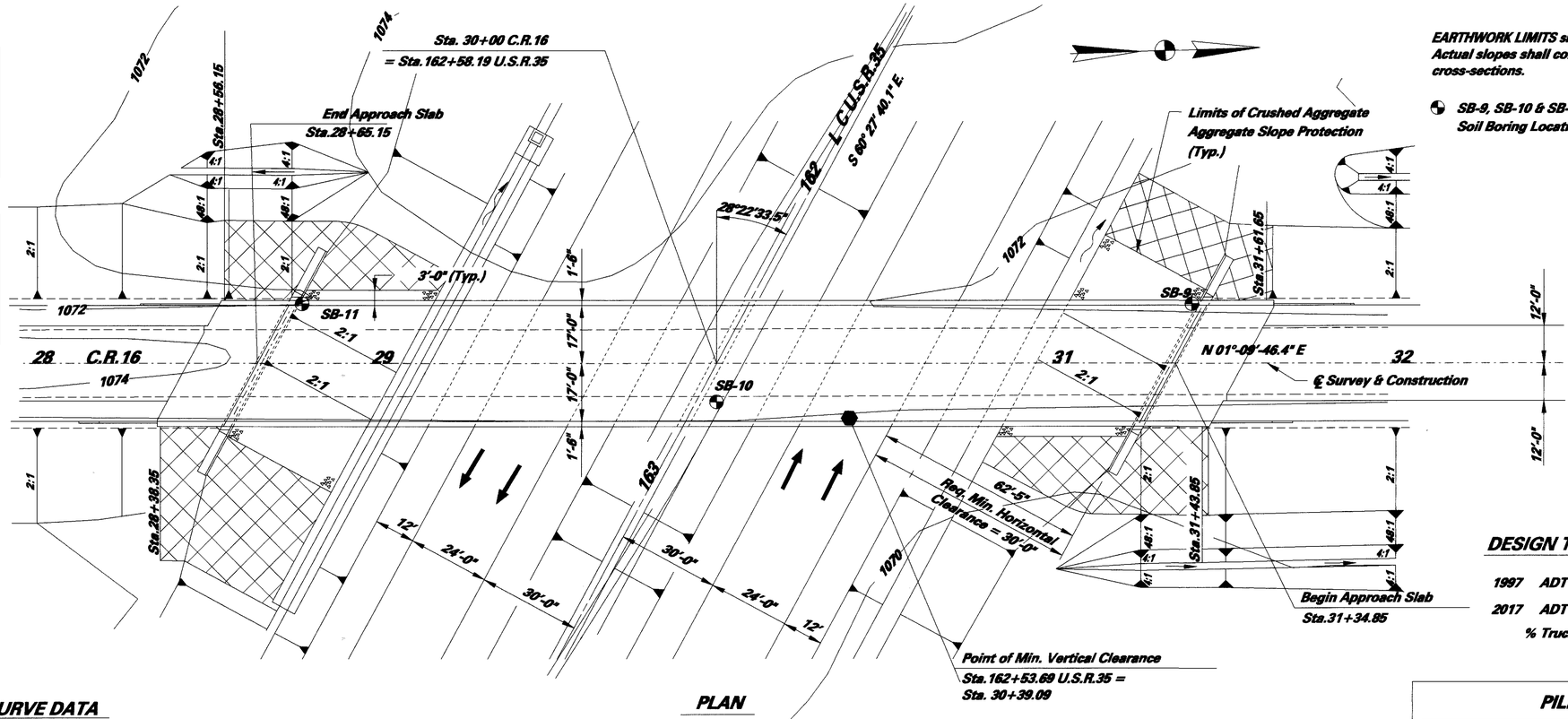
403
522

B.M.-TR200

Iron Pin With Cap
Sta.19+98.25, 13.28' Left
Elev. = 1071.69

B.M.-TR201

Iron Pin With Cap
Sta. 30+01.34, 14.24' Left
Elev. = 1073.17



PLAN

EARTHWORK LIMITS shown are approximate. Actual slopes shall conform to plan cross-sections.

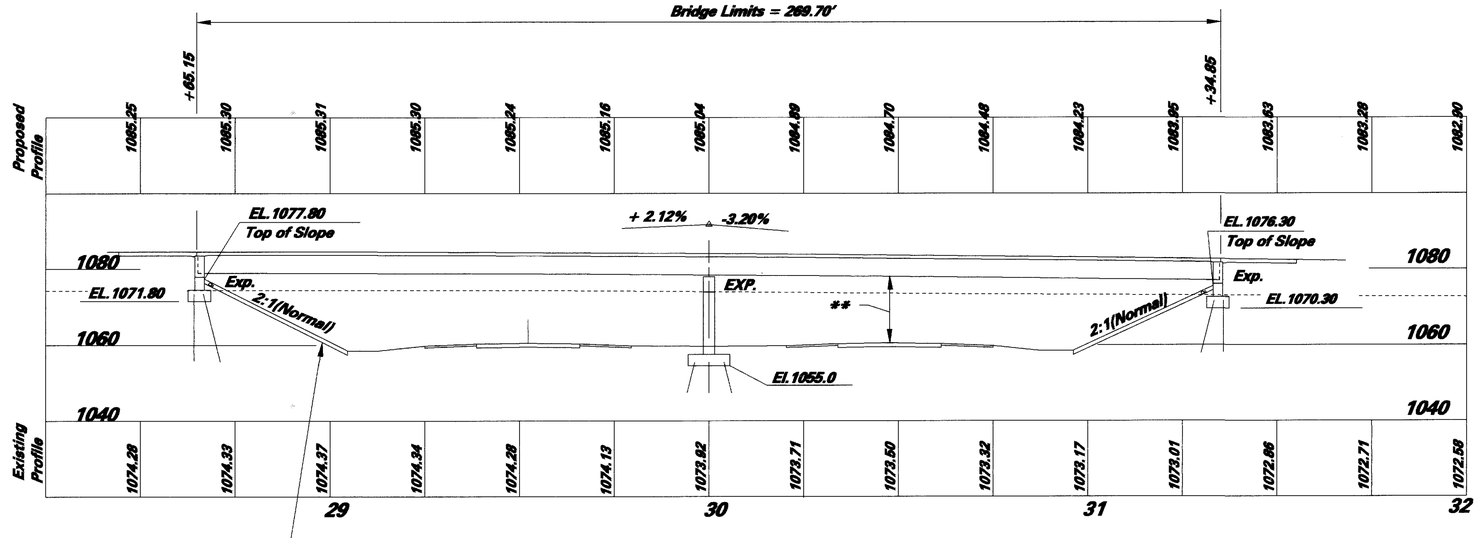
SB-9, SB-10 & SB-11 Indicate Soil Boring Locations.

DESIGN TRAFFIC

1997 ADT 980
2017 ADT 1370
% Trucks

VERTICAL CURVE DATA

P.V.I. Sta. 30+00.00
P.V.I. EL. 1091.69
G_1 = +2.12%
G_2 = -3.20%
L = 1000'



Crushed Aggregate Slope Protection, 1'-0" Thick (Typ.) Included with Roadway Quantities for Payment.

** Required Min. Vertical Clearance = 16.5'
Actual Min. Vertical Clearance = 16.96'

PROFILE
On C Survey & Construction

PILE DATA

Location	Type	Estimated Pile Length
Abutments	12"0 Conc.	45
Pier	14"0 Conc.	50

PROPOSED STRUCTURE

TYPE: Continuous Plate Girder (A572, Painted) on Reinforced Concrete Substructure
SPAN: 2 Spans @ 134'-0" c/c Bearings
ROADWAY: 34'-0" Toe to Toe of Parapets
LOADING: HS20-44(Case II) And The Alternate Military Loading
SKEW: 28°-22'-33.5"L.F.
WEARING SURFACE: Monolithic Concrete
APPROACH SLABS: 20' Long (Special)
ALIGNMENT: Tangent
CROWN : 0.0156:1
LATITUDE : 39°-37'-56"N
LONGITUDE : 83°-37'-00"W
USGS Quadrangle : Jeffersonville

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1923 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO.2400324	
SP			

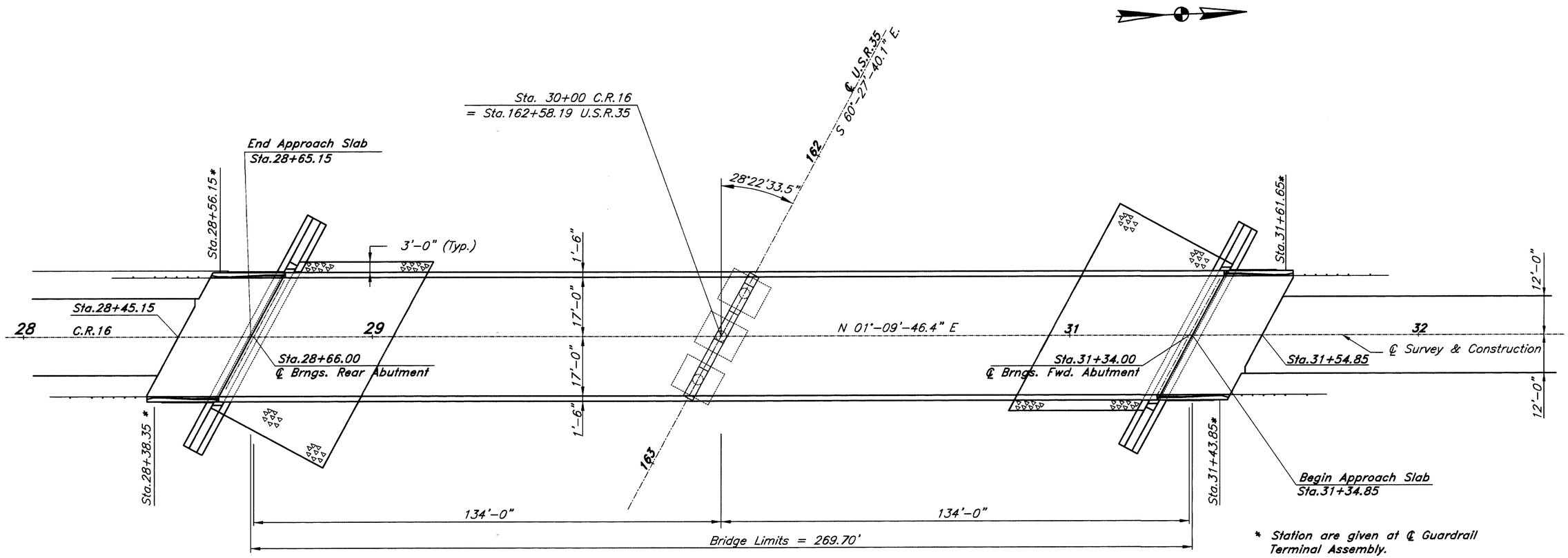
FAYETTE COUNTY
STA.28+65.15
STA.31+34.85

SITE PLAN
BRIDGE NO. FAY-35-0308
CR 16 OVER US 35

FAY-35-2.57

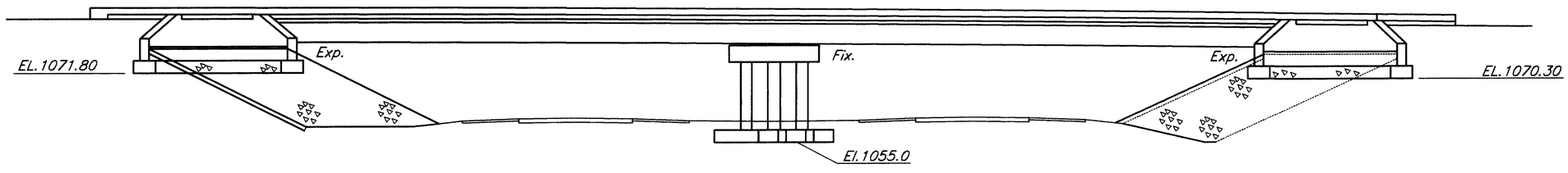
1 / 13

404
522



* Station are given at ℄ Guardrail Terminal Assembly.

GENERAL PLAN



ELEVATION
Piles not shown

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED	DATE
CEE	2-24-99

STRUCTURE FILE NO. 2400324

DRAWN	REVISED
SP	

DESIGNED	CHECKED
SP	RT

GENERAL PLAN & ELEVATION
BRIDGE NO. FAY-35-0308
CR 16 OVER US 35

FAY-35-2.57

2 / 13

405
522

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844	-	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C	-	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL	-	ASTM A615, A616 OR A617 GRADE 60
		UNIT STRESS 24,000 PSI
STRUCTURAL STEEL	-	ASTM A572-YIELD STRENGTH 50,000 P.S.I.

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81 DATED 9-15-94 SHEET NO. 1. AND 2.
BR-1 DATED 1-06-99
GSD-1-96 DATED 2-12-97 SHEET NO. 1. AND 3.

AND TO SUPPLEMENTAL SPECIFICATIONS :

842 DATED 1-8-99
844 DATED 1-6-99
863 DATED 10-12-99
816 DATED 4-21-97

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, TOP & BOTTOM MATS 2 1/2" CONCRETE COVER TOP, 1 1/2" COVER BOTTOM AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 72 TONS PER PILE FOR THE 12" 0 ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 100 TONS PER PILE FOR THE 14" 0 CIP PIER PILES.

ABUTMENT PILES:
32 PILES 45 FEET LONG, ESTIMATED LENGTH
32 PILES OF ORDER LENGTH 50 FEET LONG
10 SPLICES

PIER PILES:
27 PILES 50 FEET LONG, ESTIMATED LENGTH
27 PILES OF ORDER LENGTH 55 FEET LONG
13 SPLICES

ABUTMENT BACKFILL

ABUTMENT BACKFILL ABOVE THE BEAM SEATS SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.08 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

CONCRETE PARAPETS:

WITHIN 48 HOURS AFTER PLACEMENT OF PARAPET CONCRETE SAWCUT 1 INCH DEEP JOINTS INTO THE CONCRETE PARAPET AT LOCATIONS AS DETAILED IN THE PLANS. THE SAW CUT SHALL BE MADE IN THE COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK, AND THE COMPLETED SAWCUT SHALL BE FILLED WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION TT-S-00227E.

ITEM 203 EMBANKMENT AS PER PLAN

ALL FILL MATERIAL FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT PLACED BETWEEN STATIONS 27+50 TO 32+50 AND FOR FILLING THE EXCAVATION VOID CREATED BY ITEM 503 SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

ITEM 842 CLASS C CONCRETE, ABUTMENT, AS PER PLAN

INSTALL A 3 FOOT WIDE STRIP, 3/32 INCH THICK, GENERAL PURPOSE HEAVY DUTY NEOPRENE SHEET WITH NYLON FABRIC REINFORCEMENT AT LOCATIONS SHOWN IN THE PLANS. SECURE THE 3 FOOT WIDE NEOPRENE SHEETING TO THE CONCRETE WITH 1 1/4" X 3/32" X 1/4" (LENGTH X SHANK DIAMETER X HEAD DIAMETER) #10 GALVANIZED SCREWS THROUGH A 1 INCH OUTSIDE DIAMETER, #10 GAGE GALVANIZED WASHER. MAXIMUM FASTENER SPACING IS 9 INCHES. OTHER SIMILAR GALVANIZED DEVICES WHICH WILL NOT DAMAGE EITHER THE NEOPRENE OR THE CONCRETE MAY BE USED SUBJECT TO THE APPROVAL OF THE ENGINEER.

CENTER THE NEOPRENE STRIPS ON ALL JOINTS. FOR HORIZONTAL JOINTS, SECURE THE HORIZONTAL NEOPRENE STRIP BY USING A SINGLE LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE TOP OF THE NEOPRENE STRIP. FOR THE VERTICAL JOINTS SECURE THE VERTICAL NEOPRENE STRIP BY USING A SINGLE VERTICAL LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE VERTICAL EDGE OF THE NEOPRENE STRIP NEAREST TO THE CENTERLINE OF ROADWAY. FOR VERTICAL JOINTS, INSTALL 2 ADDITIONAL FASTENERS AT 6 INCHES CENTER TO CENTER ACROSS THE TOP HALF OF THE NEOPRENE STRIP ON THE SIDE OF TH NEOPRENE STRIP AS THE SINGLE VERTICAL ROW OF FASTENERS.

THE VERTICAL NEOPRENE STRIPS SHOULD COMPLETELY OVERLAP THE HORIZONTAL STRIPS DUE TO MATERIAL MANUFACTURING SHALL BE AT LEAST ONE FOOT IN LENGTH, IF NOT VULCANIZED OR ADHESIVED, OR 6 INCHES IN LENGTH IF THE LAP IS VULCANIZED OR ADHESIVED. NO LAPS ARE ACCEPTABLE IN VERTICALLY INSTALLED NEOPRENE STRIPS.

PAYMENT FOR LABOR, MATERIALS AND INSTALLATION OF THESE ITEMS SHALL BE INCLUDED IN ITEM 842 CLASS C CONCRETE, ABUTMENT, AS PER PLAN.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 5/13 , 6/13 , 7/13 AND 8/13 .
SEE PROPOSAL NOTE.

ITEM 503, UNCLASSIFIED EXCAVATION ***, AS PER PLAN:

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN 6 INCH LIFTS AND COMPACTED IN ACCORDANCE WITH 304.04.

DESIGN AGENCY
ERMCBSBON ENGINEERING LTD.
1520 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
VK			

STRUCTURE FILE NO. 2400324

GENERAL NOTES
BRIDGE NO. FAY-35-0308
C.R.16 OVER U.S.R.35

FAY-35-2.57

ESTIMATED QUANTITIES

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	ABUT.	PIER	SUPER.	GEN.
503	21101	386	CU YD	UNCLASSIFIED EXCAVATION, AS PER PLAN	293	93		
505	11100	LUMP	LUMP	PILE DRIVING EQUIPMENT MOBILIZATION				LUMP
507	00500	1440	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES DRIVEN	1440			
507	00550	1600	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES FURNISHED	1600			
507	00600	1350	LIN.FT.	14" CAST-IN-PLACE REINFORCED CONCRETE PILES DRIVEN		1350		
507	00650	1485	LIN.FT.	14" CAST-IN-PLACE REINFORCED CONCRETE PILES FURNISHED		1485		
507	50500	23	EACH	STEEL PILE SPLICES	10	13		
SPECIAL	51267500	724	SQ YD	SEALING OF CONCRETE SURFACES	87	101	536	
516	13100	79	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	79			
516	44101	10	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.	10			
				10" x 17" x 2 1/4" NEOPRENE WITH 1 1/2" x 11" x 18"				
				STEEL LOAD PLATE, AS PER PLAN *				
516	44101	5	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.		5		
				17" x 25" x 2 1/4" NEOPRENE WITH 2" x 18" x 32" STEEL				
				LOAD PLATE, AS PER PLAN *				
518	21200	90	CU YD	POROUS BACKFILL WITH FILTER FABRIC	90			
518	40000	145	LIN FT	6" PERFORATED CORRUGATED PLASTIC PIPE	145			
518	40010	84	LIN FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE	84			
				INCLUDING SPECIALS				
816	00600	LUMP	SUM	FIELD PAINTING OF NEW STEEL, INTERMEDIATE AND FINISH COAT,				
				SYSTEM IZEU				
844	48000	291	CU YD	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE (DECK)			291	
844	48020	72	CU YD	HIGH PERFORMANCE CONCRETE SUPERSTRUCTURE (PARAPET)			72	
844	49000		LUMP	HIGH PERFORMANCE CONCRETE TRIAL MIX				
844	49010		LUMP	HIGH PERFORMANCE CONCRETE TESTING				
842	41000	31	CU YD	CLASS C CONCRETE, PIER CAPS AND COLUMN		31		
842	44101	70	CU YD	CLASS C CONCRETE, ABUTMENT ABOVE FOOTING, AS PER PLAN	70			
842	46500	144	CU YD	CLASS C CONCRETE, FOOTING	97	47		
863	10080	LUMP	SUM	STRUCTURAL STEEL, MEMBERS LEVEL FOUR (4) FABRICATION				
863	20000	1995	EACH	WELDED STUD SHEAR CONNECTORS			1995	
				Computed by: S.P. Date: 6-8-94				
				Checked by: V.K. Date: 7-8-94				

* SEE PROPOSAL NOTE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

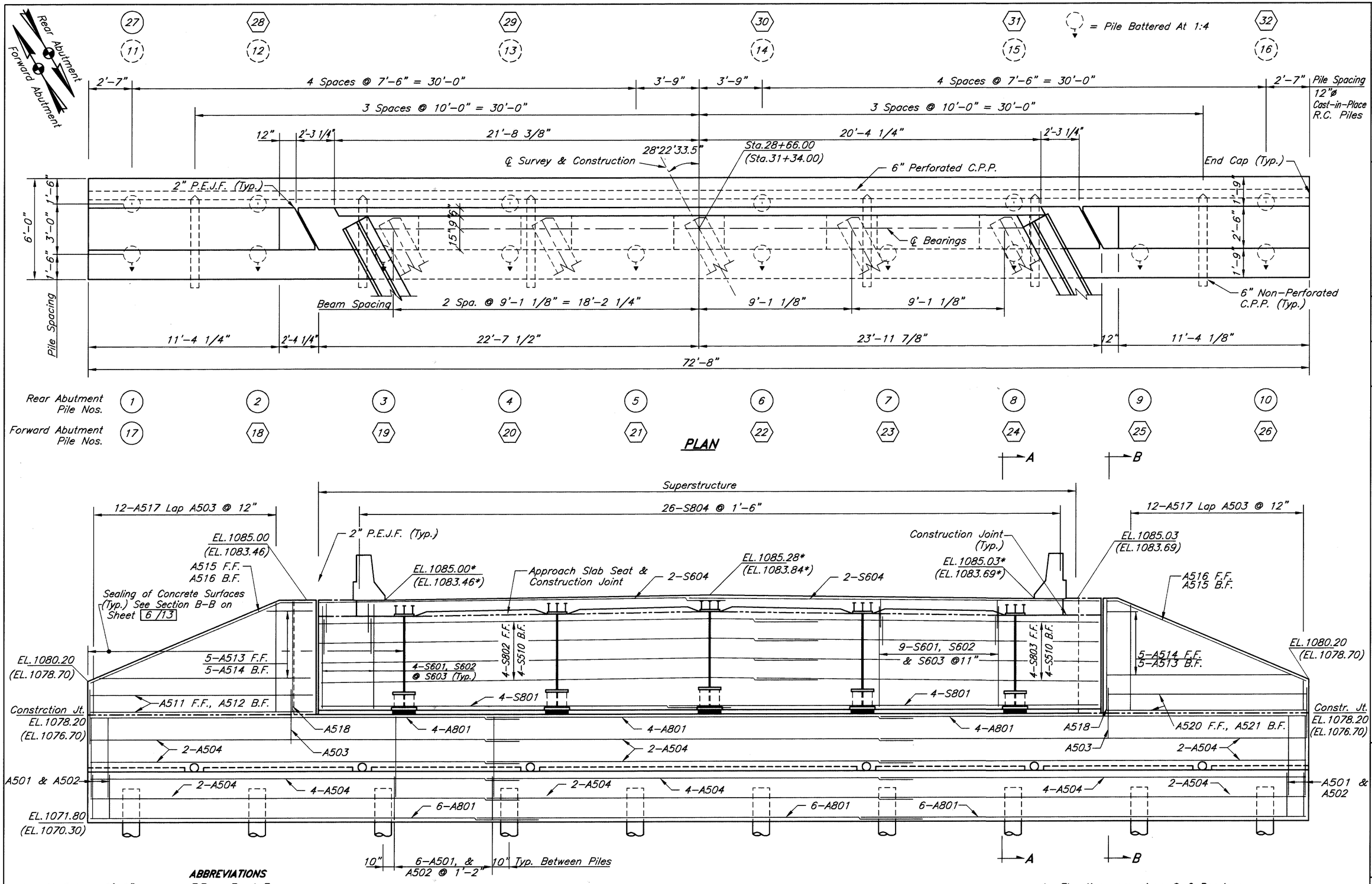
CHECKED VK | REVISED

STRUCTURE FILE NO. 2400324

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0308
C.R.16 OVER U.S.R.35

FAY-35-2.57

Revised 8/11/00 by E.R.P.



PLAN

ELEVATION

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Plastic Pipe
P.E.J.F. = Preformed Expansion Joint Filler

Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

* Elevations are given @ ℄ Bearings

Elevations & Station in ( ) are for Forward Abutment only.

For notes,and Sections A-A & B-B see sheet 6/13

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99

CHECKED SP/VK | REVISED

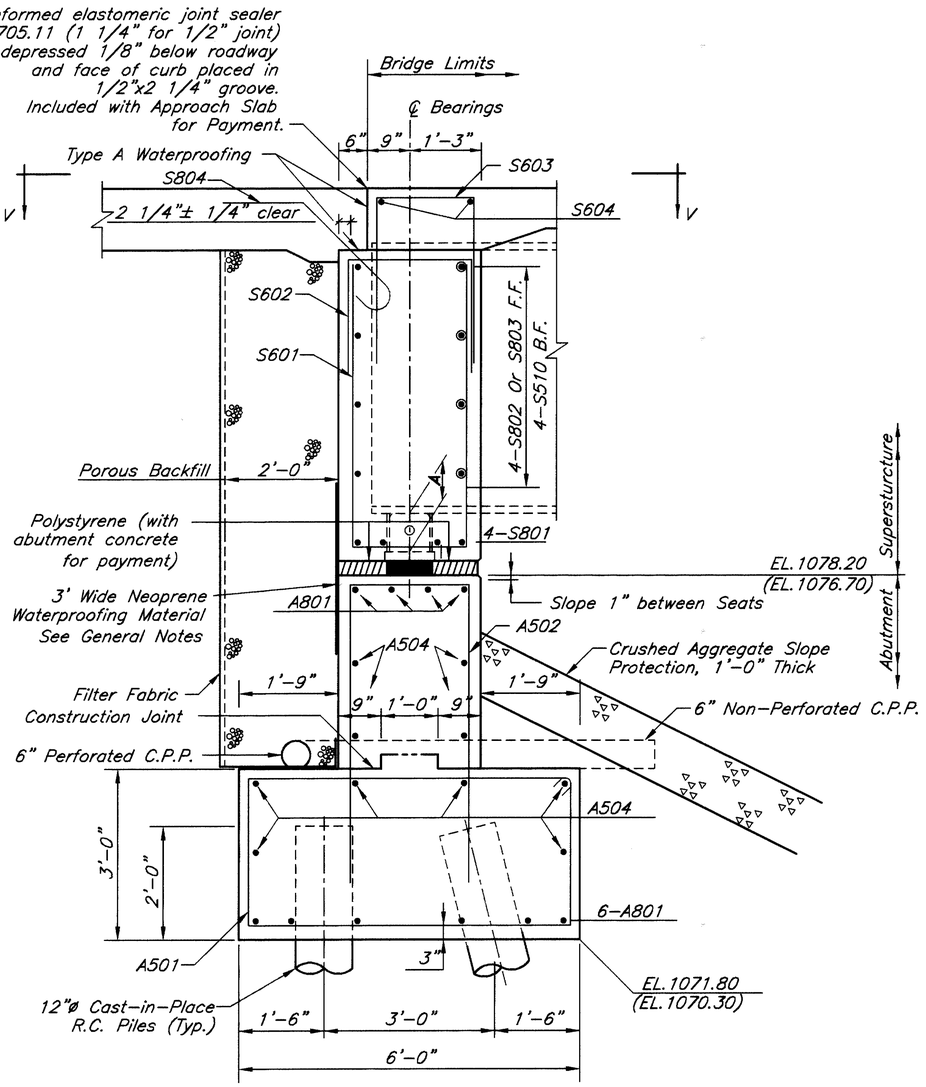
STRUCTURE FILE NO.2400324

ABUTMENT DETAILS
Bridge No. FAY-35-0308
C.R.16 OVER US 35

FAY-35-2.57

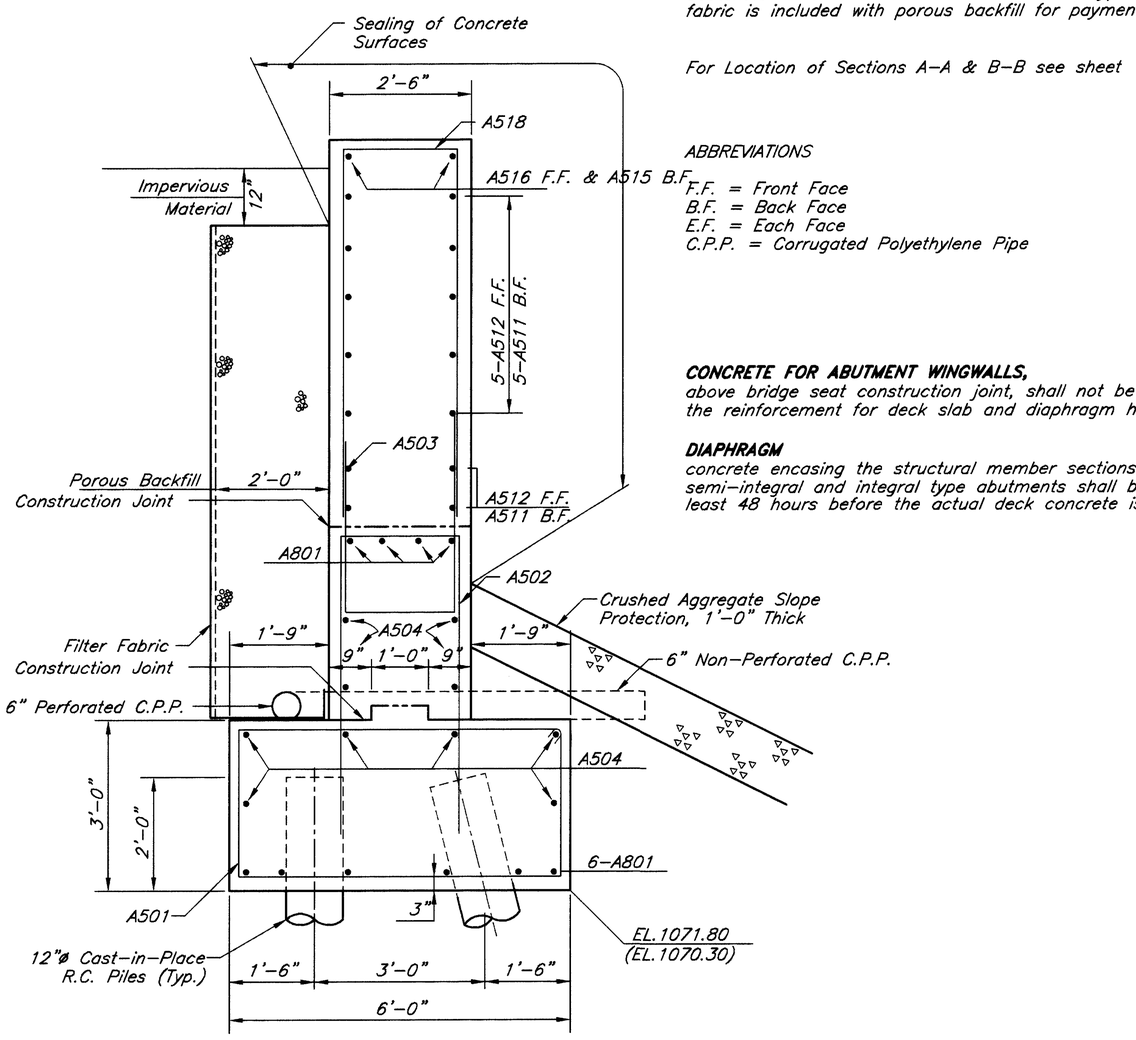
5/13

408
522

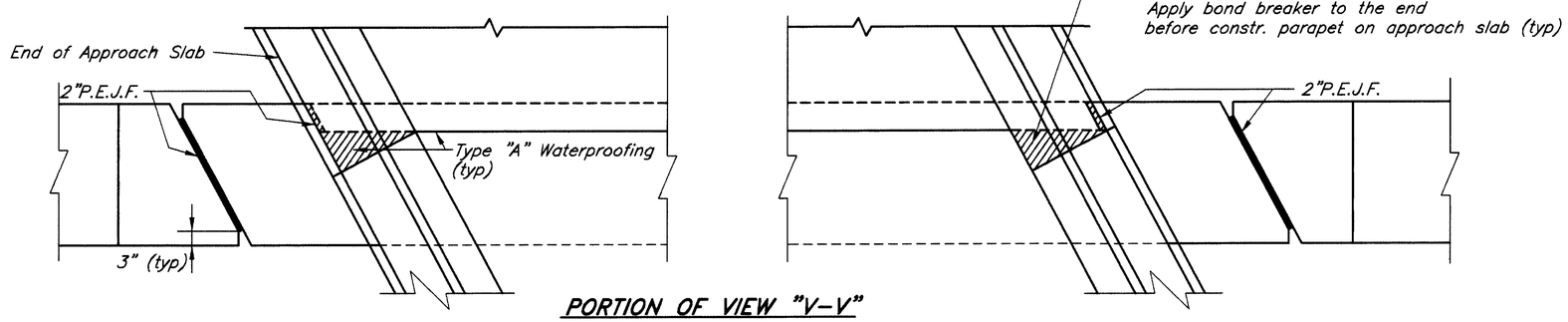


SECTION A-A

For Diminsion "A" & Beam Support Details
See Sheet 12 | 13



SECTION B-B



PORTION OF VIEW "V-V"

POROUS BACKFILL with filter fabric shall extend upward to the plane of the subgrade, to one foot below the embankment surface, and laterally to the ends of the wingwalls. Geotextile fabric shall conform with 712.09, Type A. Geotextile fabric is included with porous backfill for payment.

For Location of Sections A-A & B-B see sheet 5 | 13

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Polyethylene Pipe

CONCRETE FOR ABUTMENT WINGWALLS,
above bridge seat construction joint, shall not be placed until the reinforcement for deck slab and diaphragm has been erected.

DIAPHRAGM
concrete encasing the structural member sections supported in semi-integral and integral type abutments shall be placed at least 48 hours before the actual deck concrete is placed.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

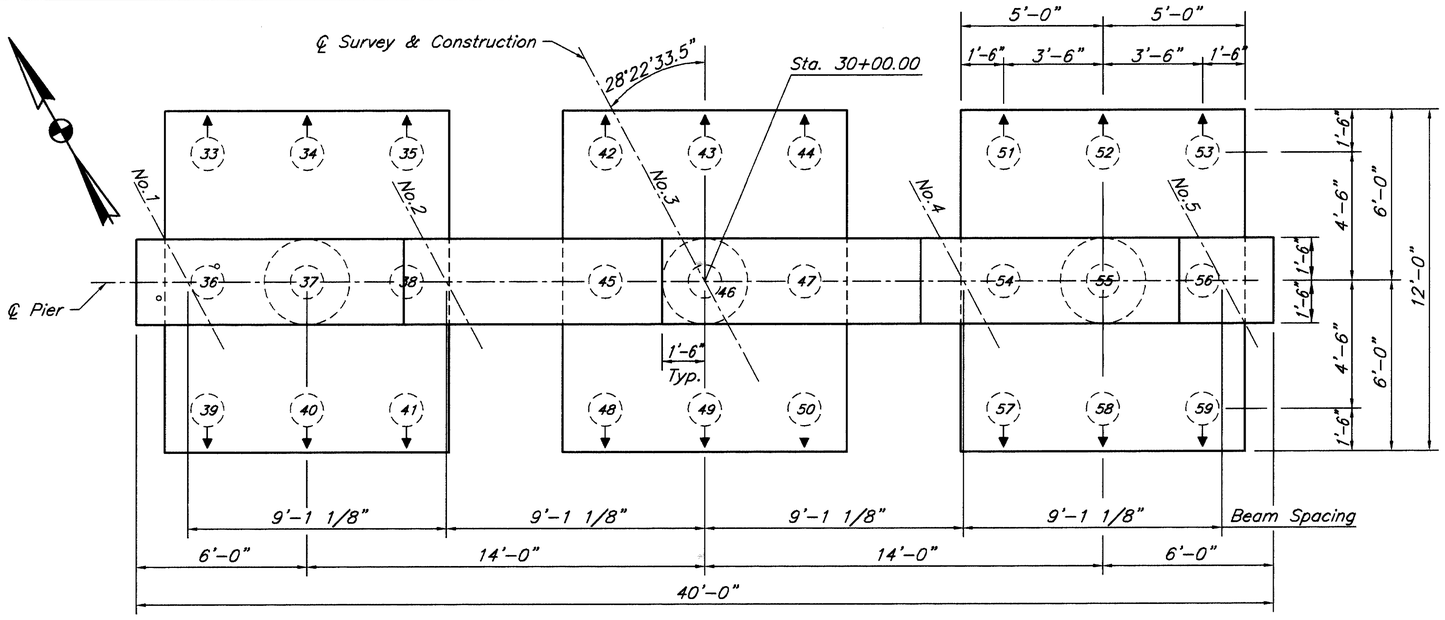
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400324

ABUTMENT DETAILS

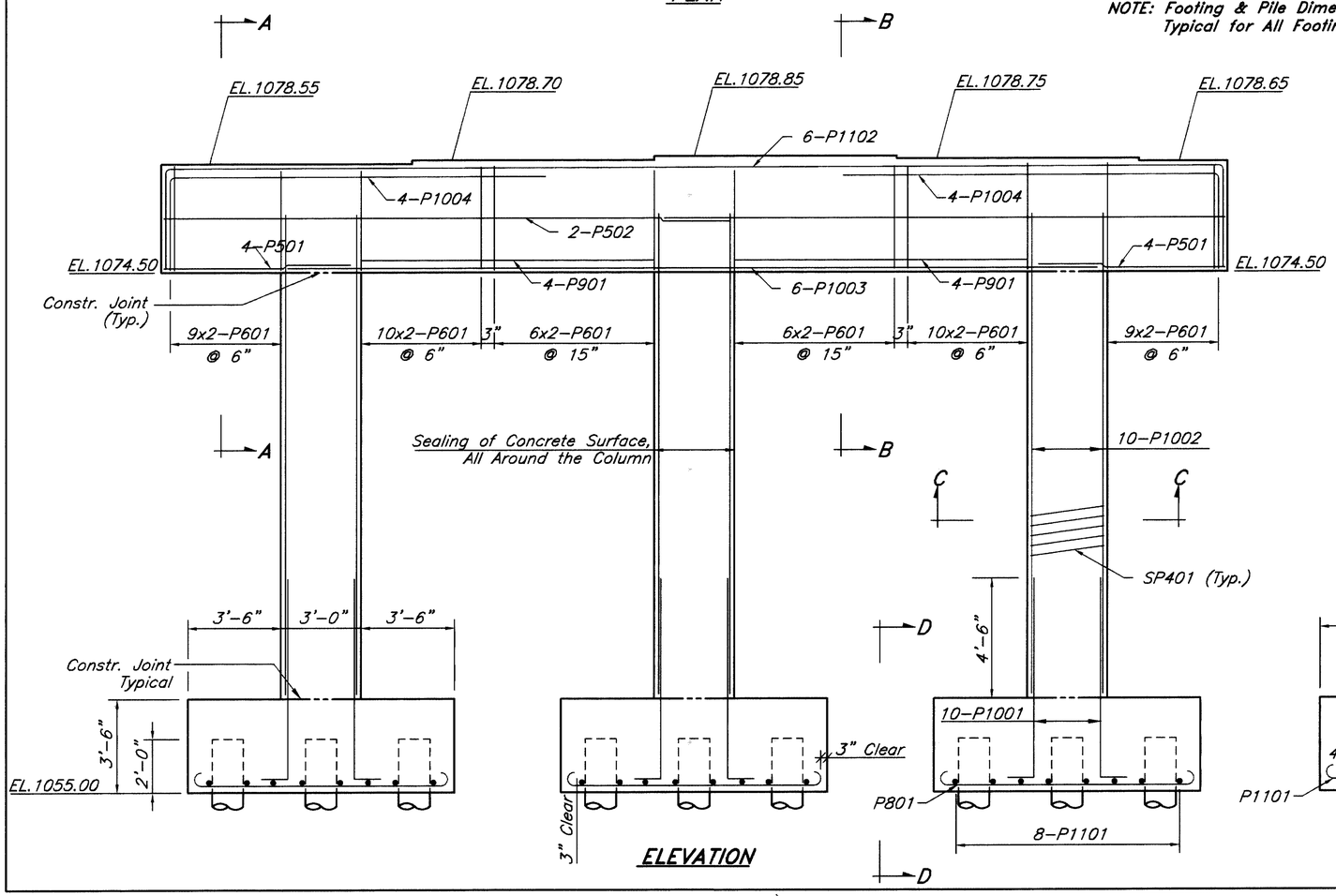
Bridge No. FAY-35-0308
C.R.16 OVER US 35

FAY-35-2.57

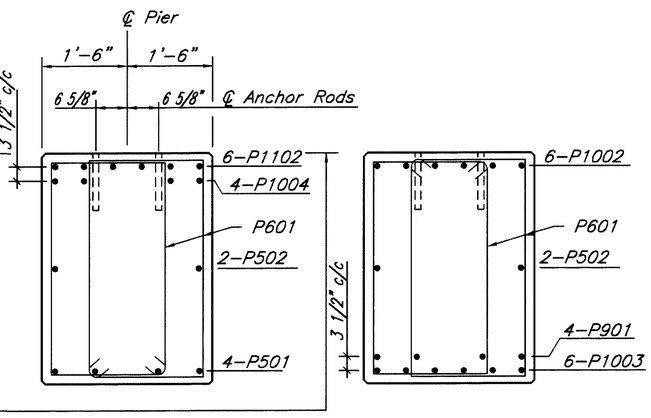


PLAN

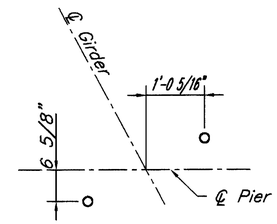
NOTE: Footing & Pile Dimensions are Typical for All Footings.



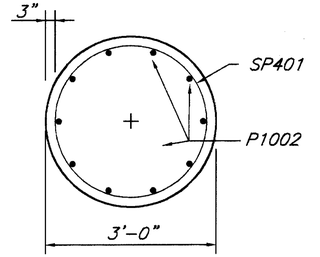
ELEVATION



SECTION A–A

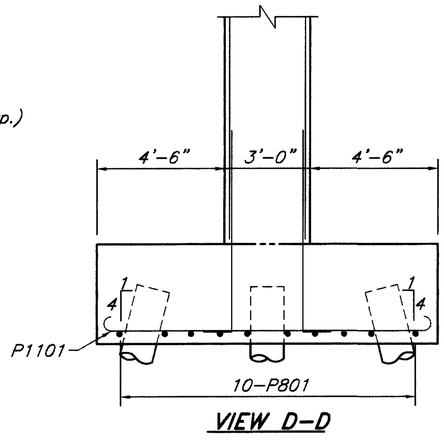


SECTION B–B



BEARING ANCHORS LAYOUT

SECTION C–C



VIEW D–D

BRIDGE SEAT REINFORCING:

REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT SHALL BE ACCURATELY PLACED TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE–SETTING OF BEARING ANCHORS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2–24–99
CHECKED	REVISED		
SP/VK			

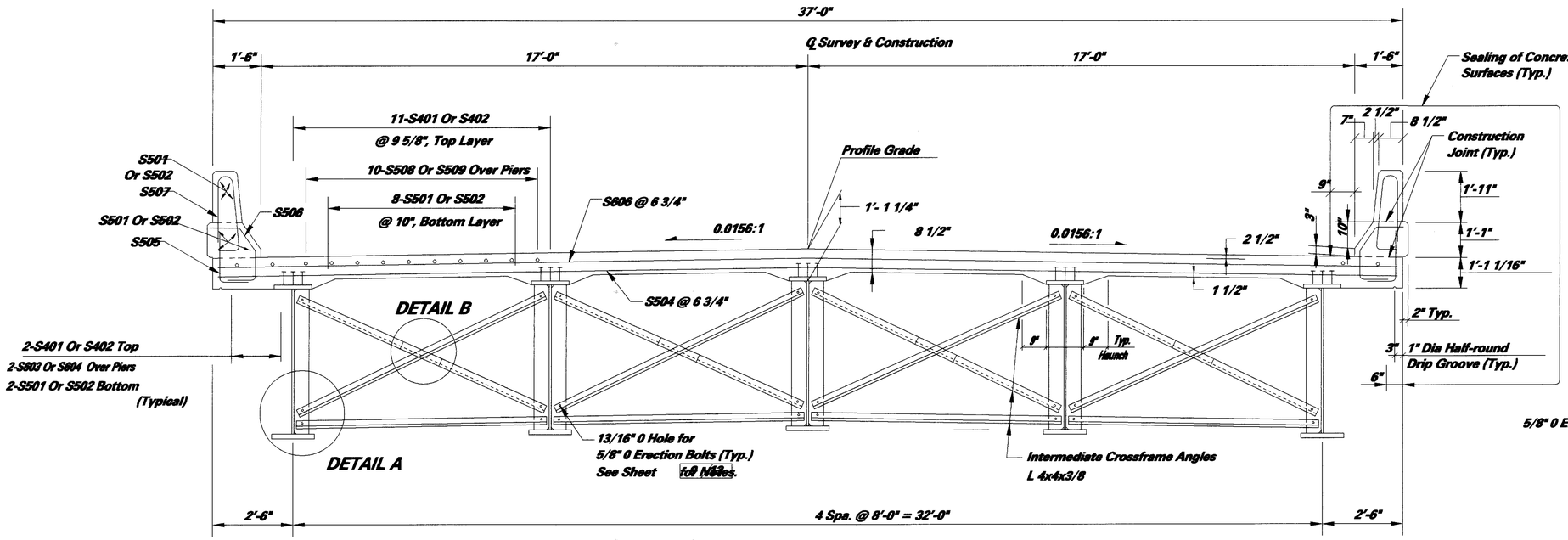
STRUCTURE FILE NO.30002I4

PIER DETAILS
Bridge No. FAY–35–0308
C.R.16 OVER US 35

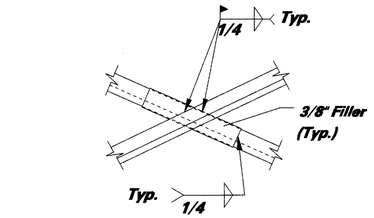
FAY–35–2.57

7 / 13

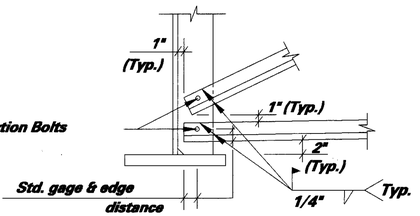
410
522



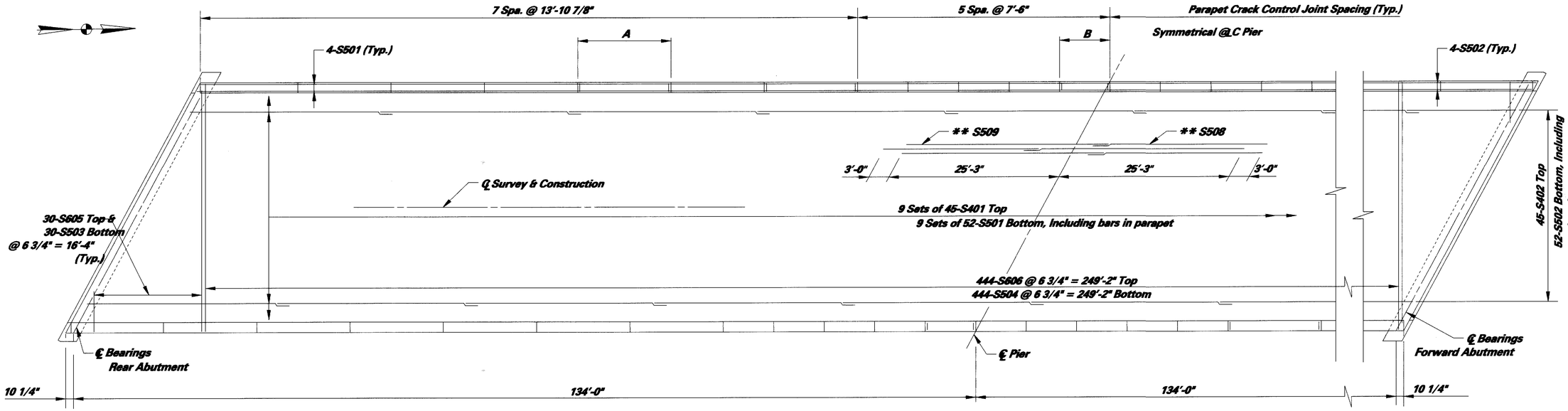
TRANSVERSE SECTION



DETAIL B



DETAIL A



DECK REINFORCING PLAN

A = 11-S505, S506 & S507 @ 1'-4"

B = 6-S505, S506 & S507 @ 1'-5"

Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

** 44-S508 and 44-509 required over pier

For Notes See Sheet 9/13

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
Structure File No.2400324

DRAWN RT
REVISED

DESIGNED RT
CHECKED SP/VK

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0308
CR 16 OVER US 35

FAY-35-2.57

SCREED ELEVATION TABLE

		SCREED LINE 1		SCREED LINE 2		SCREED LINE 3		SCREED LINE 4		SCREED LINE 5	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L ≠	28+75.18	1085.03	28+70.32	1085.17	28+66.00	1085.29	28+61.68	1085.15	28+56.82	1085.00
	0.10 L	28+88.58	1085.10	28+83.72	1085.24	28+79.40	1085.36	28+75.08	1085.23	28+70.22	1085.08
	0.20 L	29+1.98	1085.16	28+97.12	1085.30	28+92.80	1085.41	28+88.48	1085.29	28+83.62	1085.14
	0.30 L	29+15.38	1085.19	29+10.52	1085.33	29+6.20	1085.46	29+1.88	1085.33	28+97.02	1085.19
	0.40 L	29+28.78	1085.18	29+23.92	1085.33	29+19.60	1085.46	29+15.28	1085.34	29+10.42	1085.20
	0.50 L	29+42.18	1085.14	29+37.32	1085.30	29+33.00	1085.43	29+28.68	1085.32	29+23.82	1085.17
	0.60 L	29+55.58	1085.08	29+50.72	1085.24	29+46.40	1085.37	29+42.08	1085.26	29+37.22	1085.12
	0.70 L	29+68.98	1085.00	29+64.12	1085.15	29+59.80	1085.28	29+55.48	1085.18	29+50.62	1085.05
	0.80 L	29+82.38	1084.92	29+77.52	1085.06	29+73.20	1085.20	29+68.88	1085.09	29+64.02	1084.98
	0.90 L	29+95.78	1084.81	29+90.92	1084.97	29+86.60	1085.12	29+82.28	1085.01	29+77.42	1084.89
	1.00 L	30+9.18	1084.72	30+4.32	1084.89	30+0.00	1085.04	29+95.68	1084.94	29+90.82	1084.82
Span No.2											
	0.10 L	30+22.58	1084.65	30+17.72	1084.82	30+13.40	1084.97	30+9.08	1084.87	30+4.22	1084.76
	0.20 L	30+35.98	1084.59	30+31.12	1084.76	30+26.80	1084.91	30+22.48	1084.82	30+17.62	1084.71
	0.30 L	30+49.38	1084.53	30+44.52	1084.69	30+40.20	1084.85	30+35.88	1084.76	30+31.02	1084.67
	0.40 L	30+62.78	1084.45	30+57.92	1084.63	30+53.60	1084.79	30+49.28	1084.70	30+44.42	1084.60
	0.50 L	30+76.18	1084.35	30+71.32	1084.54	30+67.00	1084.71	30+62.68	1084.62	30+57.82	1084.51
	0.60 L	30+89.58	1084.23	30+84.72	1084.42	30+80.40	1084.59	30+76.08	1084.51	30+71.22	1084.41
	0.70 L	31+2.98	1084.08	30+98.12	1084.28	30+93.80	1084.44	30+89.48	1084.36	30+84.62	1084.27
	0.80 L	31+16.38	1083.90	31+11.52	1084.10	31+7.20	1084.27	31+2.88	1084.18	30+98.02	1084.10
	0.90 L	31+29.78	1083.68	31+24.92	1083.88	31+20.60	1084.06	31+16.28	1083.98	31+11.42	1083.90
	1.00 L	31+43.18	1083.46	31+38.32	1083.66	31+34.00	1083.84	31+29.68	1083.77	31+24.82	1083.69

Screed elevations are for the concrete surface before concrete is placed. Allowance has been made for the anticipated deflection of the structure steel.

≠ L = Span Length

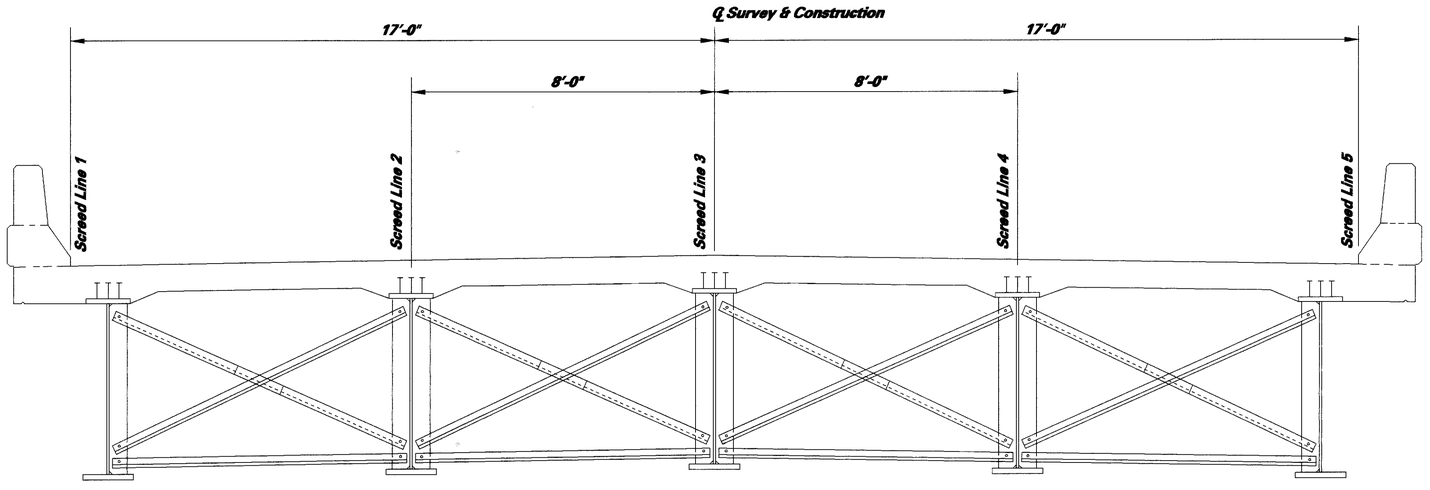
THE DISTANCE SHOWN FROM THE TOP OF THE DECK SLAB TO THE BOTTOM OF THE TOP FLANGE IS THE THEORETICAL DESIGN DIMENSION. THE QUANTITY OF DECK CONCRETE TO BE PAID FOR SHALL BE BASED UPON THIS DIMENSION, EVEN THOUGH DEVIATION FROM IT MAY BE NECESSARY BECAUSE THE TOP FLANGE OF THE GIRDER MAY NOT HAVE THE EXACT CAMBER OR CONFORMATION REQUIRED TO PLACE IT PARALLEL TO THE FINISHED GRADE. DEDUCTION SHALL BE MADE FOR VOLUME OF ENCASED STEEL PLATES AS PER 842.

CONCRETE DECK HAUNCH WIDTH

A HAUNCH WIDTH OF 9 INCHES SHALL BE USED FOR COMPUTING QUANTITY OF CONCRETE. HOWEVER, THE HAUNCH WIDTH MAY VARY BETWEEN 6 AND 12 INCHES.

ERECTION BOLTS: THE HOLE DIAMETER IN THE CROSS FRAMES AND GIRDER STIFFENERS SHALL BE 3/16" LARGER THAN THE DIAMETER OF THE ERECTION BOLTS. UNLESS REPLACED BY PERMANENT HIGH STRENGTH BOLTS, ERECTION BOLTS SHALL REMAIN IN PLACE. LOCK WASHERS SHALL BE FURNISHED FOR OTHER THAN FULLY TORQUED HIGH STRENGTH ERECTION BOLTS. BOLTS SHALL BE FURNISHED AS PART OF ITEM 863.

IN LIEU OF ERECTION BOLTS AND AT THE OPTION OF THE CONTRACTOR, ALTERNATIVE MEANS OF TEMPORARY BRACING MAY BE USED SUBJECT TO THE APPROVAL OF THE DIRECTOR (501.06).



TRANSVERSE SECTION

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

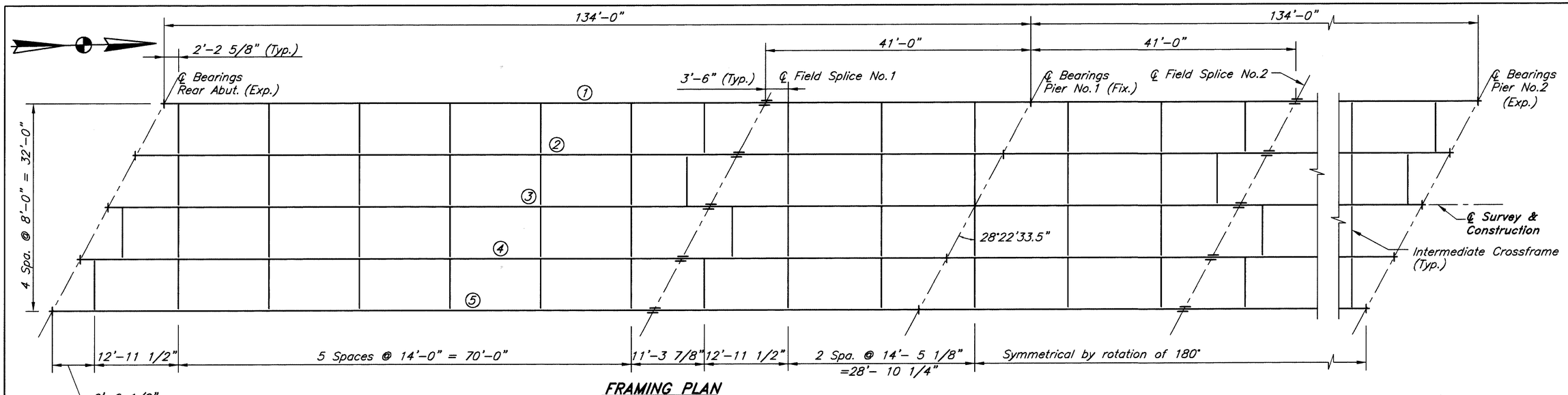
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.9000214

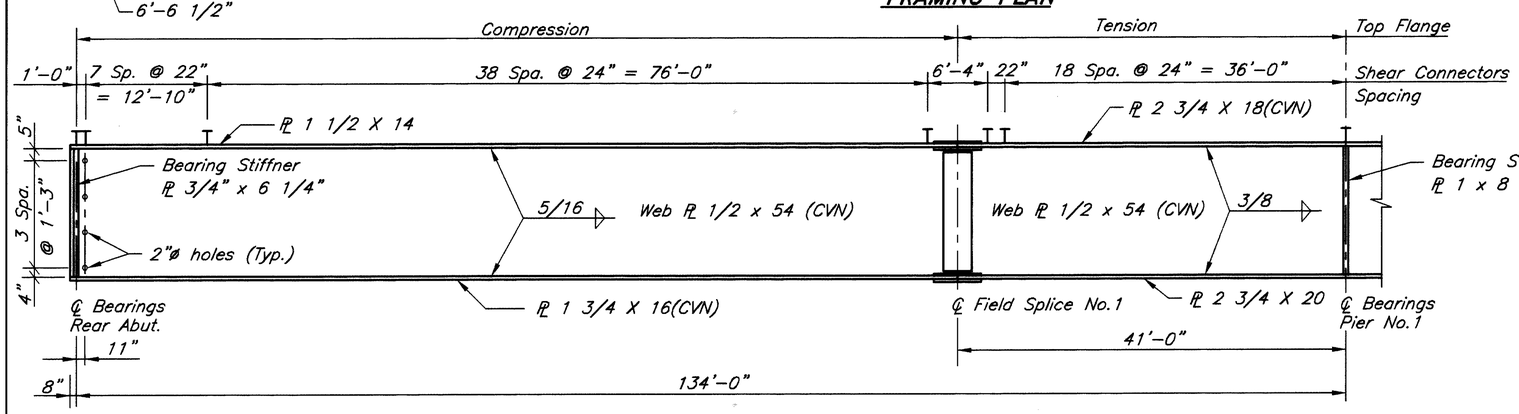
SCREED ELEVATIONS
Bridge No. FAY-35-0308
CR 16 Over US 35

FAY-35-2.57

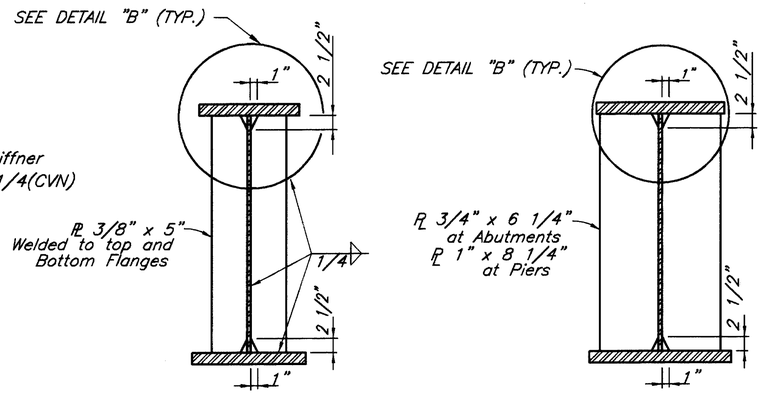
412
522



FRAMING PLAN



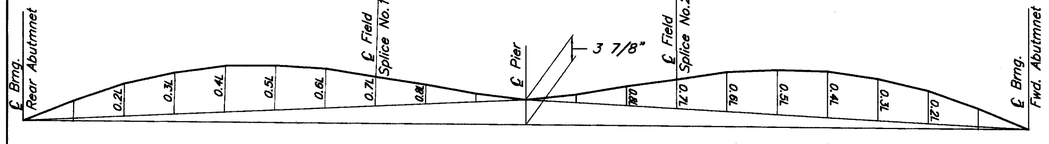
HALF-GIRDER ELEVATION



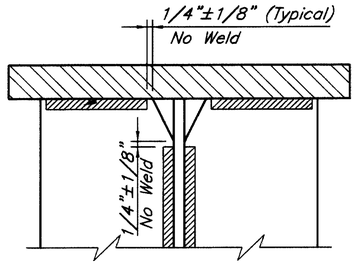
CROSSFRAME CONNECTION PLATE DETAIL

BEARING STIFFENER DETAIL

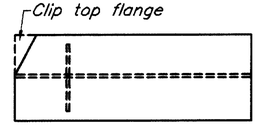
DEFLECTION AND CAMBER										
	Points	Span No.1								
		.10	.20	.30	.40	.50	.60	.70	.80	.90
ALL GIRDERS	Deflection due to weight of steel	1/4	7/16	9/16	5/8	9/16	1/2	5/16	3/16	1/16
	Deflection due to remaining dead load	3/4	1 3/8	1 3/4	1 7/8	1 3/4	1 7/16	15/16	1/2	1/8
	Adjustment for Vertical curve	1/2	15/16	1 3/16	1 3/8	1 7/16	1 3/8	1 3/16	15/16	1/2
	Net Total Camber	1 1/2"	2 3/4"	3 1/2"	3 7/8"	3 3/4"	3 5/16"	2 7/16"	1 5/8"	11/16"



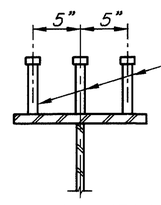
CAMBER DIAGRAM



DETAIL "B"



GIRDER FLANGE DETAIL



SHEAR CONNECTOR DETAIL

NOTES :

WELDED ATTACHMENT *of supports for concrete deck finishing machine may be made to areas of the Fascia Girder flanges designated "Compression". Attachments shall not be made to areas designated "Tension" . Fillet welds to compression flanges shall be not closer than 1" from edge of the flange, be not more than 2" long and be not smaller than the minimum size required by AASHTO.*

For additional notes see sheet 11/13

For details of bearing stiffener at abutments see sheet 12/13

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO.2400324
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0308
C.R.16 OVER U.S.R.35

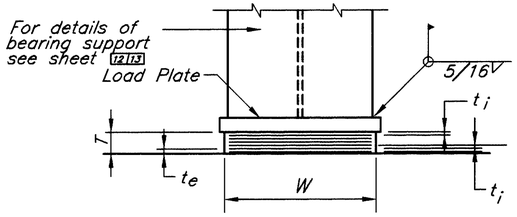
FAY-35-2.57

LAMINATED ELASTOMERIC BEARINGS

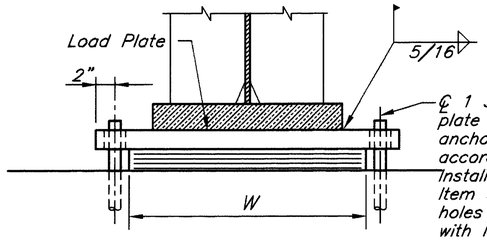
LOCATION	DIMENSIONS (Inch)					N	STEEL LOAD PLATE	REACTIONS		MAXIMUM DESIGN LOAD
	L	W	t_i	t_e	T			DL	LL	
Abutments Expansion Brngs.	10	17	0.268	0.192	2 1/4	6	11" x 18" x 1 1/2"	108 k	52 k	160 k
Pier Fixed Brngs.	17	25	0.414	0.296	2 1/4	4	18" x 32" x 2"	330 k	99 k	429 k

t_i = Thickness of Internal Layer
t_e = Thickness of External Layer
T = Total Thickness of Elastomeric Bearing

N = No. of Steel Laminates
Internal Steel Laminate Thickness = 0.0747"
Durometer of Elastomer = 50 Durometer



LAMINATED ELASTOMERIC EXPANSION BEARING



LAMINATED ELASTOMERIC FIXED BEARING

LOAD PLATE

The steel load plate shall be the same material as the attached structural steel and be similarly cleaned and coated. Surface preparation and priming shall be done in the shop and be included in the price bid for the bearings. Field coats shall be included in the price bid for painting main structural steel.

The steel load plate shall be bonded by vulcanization to the elastomer during the molding process.

Welding of the load plate to the superstructure shall be controlled so that the plate temperature at the elastomer bonded surface shall not exceed 300° F as determined by the use of pyrometric sticks or other temperature monitoring devices.

BASIS OF PAYMENT

The unit bid price shall include all materials, labor and incidentals necessary to furnish and install laminated elastomeric bearings either fixed or expansion. Payment will be made at the contract price for Item 516, each, Laminated Elastomeric Bearings.

ELASTOMERIC BEARINGS

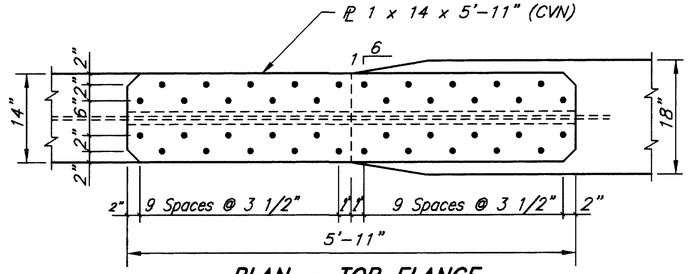
Elastomeric bearings shall comply with Item 516 and articles 18.2.5 through 18.2.8, Section 18, Bearing Devices, Division II, Construction, of the AASHTO Specifications for Highway Bridges. Bearings shall be Grade 3, 50-durometer elastomer and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the price bid for the bearings.

HIGH STRENGTH BOLTS

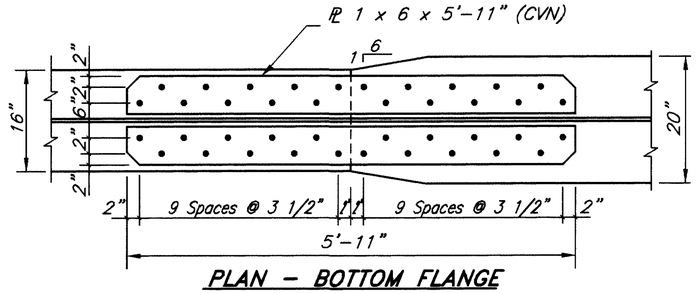
High strength bolts shall be 1" diameter A325 unless otherwise noted.

CHARPY V-NOTCH TOUGHNESS REQUIREMENT

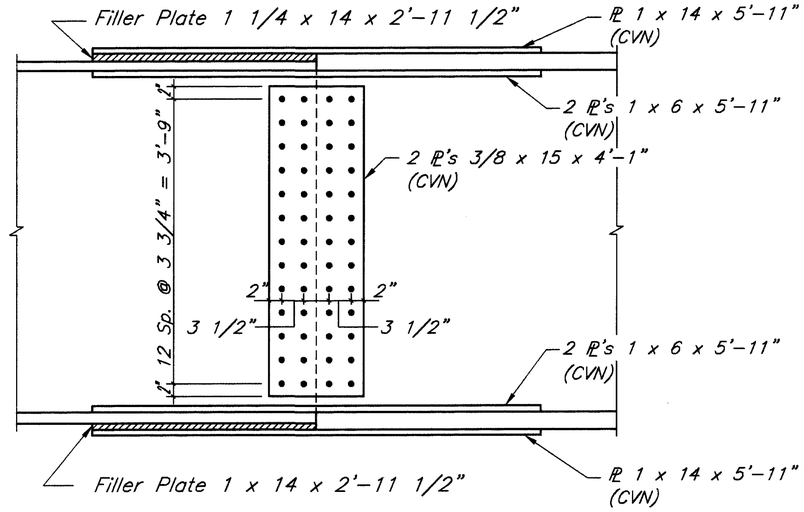
Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in 711.01 of CMS.



PLAN - TOP FLANGE



PLAN - BOTTOM FLANGE



TYPICAL FIELD SPLICE DETAILS

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

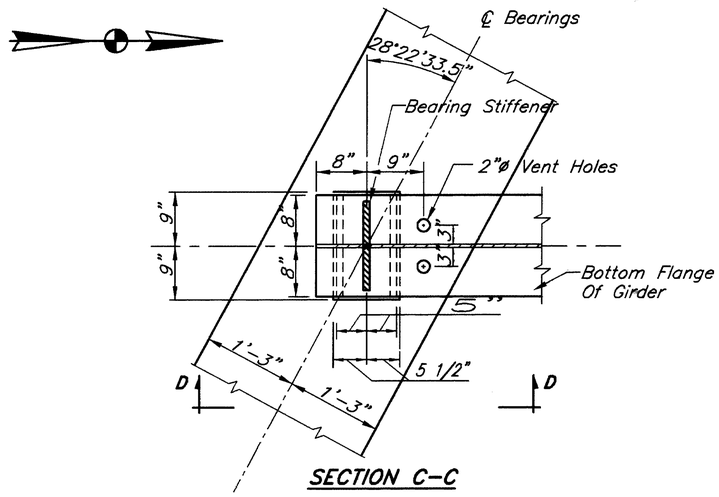
DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.2400324

BEARING & SPLICE DETAILS
Bridge No. FAY-35-0308
C.R.16 OVER US 35

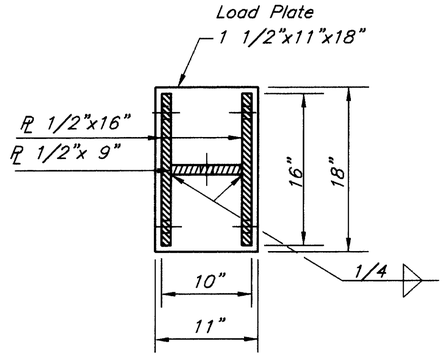
FAY-35-2.57

11 / 13

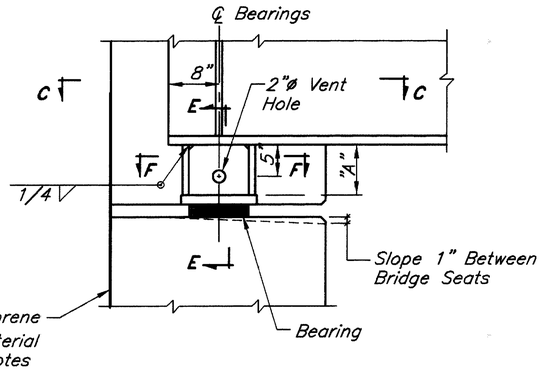
414
522



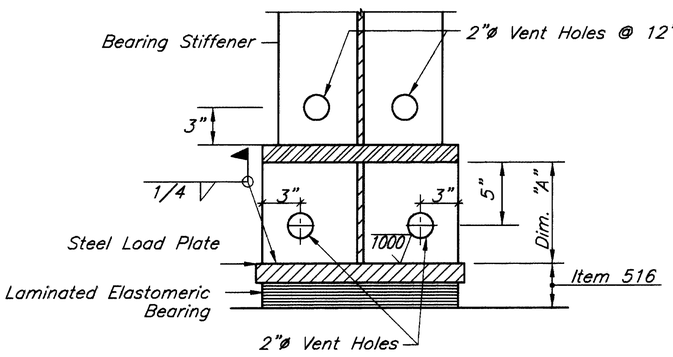
SECTION C-C



SECTION F-F



SECTION D-D



SECTION E-E

BEARING SUPPORT -DIM. "A"					
GIRDER NOS.	1	2	3	4	5
Rear Abutment	9 7/16"	10 7/8"	1'-0 1/4"	10 11/16"	9 1/16"
Forward Abutment	8 5/8"	10 3/4"	1'-0 15/16"	1'-0 1/16"	11 3/16"

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT
DRAWN RT
CHECKED VK
REVISED
REVIEWED CEE
DATE 2-24-99
STRUCTURE FILE NO. 2400324

BEARING AND JOINT SEAL DETAILS
BRIDGE NO. FAY-35-0308
C.R.16 OVER U.S.R.35

FAY-35-2.57

12 / 13

415
522

REINFORCING STEEL LIST

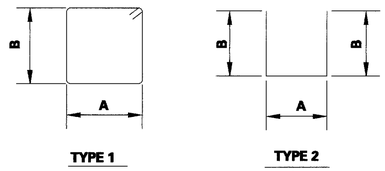
MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				ABUTMENTS					
A501	116	17-1	2067	1	5-8	2-7			
A502	116	12-5	1502	2	2-2	5-3			
A503	52	9-11	538	2	2-2	4-0			
A504	60	25-6	1596	STR					
A511	8	13-3	111	STR					
A512	8	12-0	100	STR					
A513	20	3-6 to	167	STR					4 Sets of 5
		12-6							Vary by 2'-3"
A514	20	2-2 to	139	STR					4 Sets of 5
		11-2							Vary by 2'-3"
A515	4	14-0	58	10	12-0	2-0	10-0		
A516	4	12-0	50	10	12-0	0-10	0-5		
A517	48	5-7 to	509	2	2-2	1-10 to			4 Sets of 12
		14-9				6-5			Vary "B" by 5"
A518	4	14-11	62	2	2-2	6-6			
A801	60	26-8	4272	STR					
			11171	LBS					
				PIER					
P501	8	7-6	63	STR					
P502	4	20-10	87	STR					
P601	100	12-0	1802	1	2-0 1/2	3-8			
P801	30	11-4	908	9	9-6				
P901	8	11-0	299	STR					
P1001	30	9-3	1194	4	1-10	7-9			
P1002	30	19-0	2453	STR					
P1003	6	28-0	723	STR					
P1004	8	18-0	620	4	3-2	13-8			
P1101	24	14-8	1870	9	11-6				
P1102	6	45-10	1461	2	39-8	3-5			
Spirals			918						
			12398	LBS					

Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				SUPERSTRUCTURE (AT ABUTMENT)					
S510	16	24-2	403	STR					
S601	88	12-0	1586	2	2-2	5-1			
S602	88	6-0	793	2	2-2	2-1			
S603	88	7-6	991	2	1-8	3-1			
S604	8	24-4	292	STR					
S801	16	25-0	1068	STR					
S802	8	27-9	593	STR					
S803	8	22-0	470	STR					
S804	52	4-8	648	3	2-4	1-5			
			6844	LBS					
				SUPERSTRUCTURE (DECK)					
S401	405	30-0	8116	STR					
S402	45	14-4	431	STR					
S501	468	30-0	14644	STR					
S502	52	17-4	940	STR					
S503	60	5-11to	1319	STR					2 SETS OF 30
		36-3							VARY BY 1'-0 1/2"(±)
S504	444	36-8	16980	STR					
S505	428	2-4	1042	4	10 1/2	1-7			
S506	428	3-4	1486	5	0-11				
S507	428	6-0	2678	6					
S508	44	25-6	1170	STR					
S509	44	30-0	1377	STR					
S605	60	5-11to	1900	STR					2 SETS OF 30
		36-3							VARY BY 1'-0 1/2"(±)
S606	444	36-8	24453	STR					
			76538	LBS					
					PARAPET				
					See Sheet 260				

Pitch 4 1/2"

ALL BARS SHALL BE EPOXY COATED



MARK	NO.	LENGTH	WEIGHT	SHAPE	REMARKS
SP401	3	16-2	918	BT.	
			918	LBS.	

SP401 - Core Diameter 30" c/c

Other Details in Accordance with C.R.S.I. Standard Practice.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Four steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 3.20 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED CEE | DATE 2-24-99 | STRUCTURE FILE NO.2400324

DRAWN SP | REVISED

DESIGNED SP | CHECKED VK

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0308
C.R.16 OVER U.S.R.35

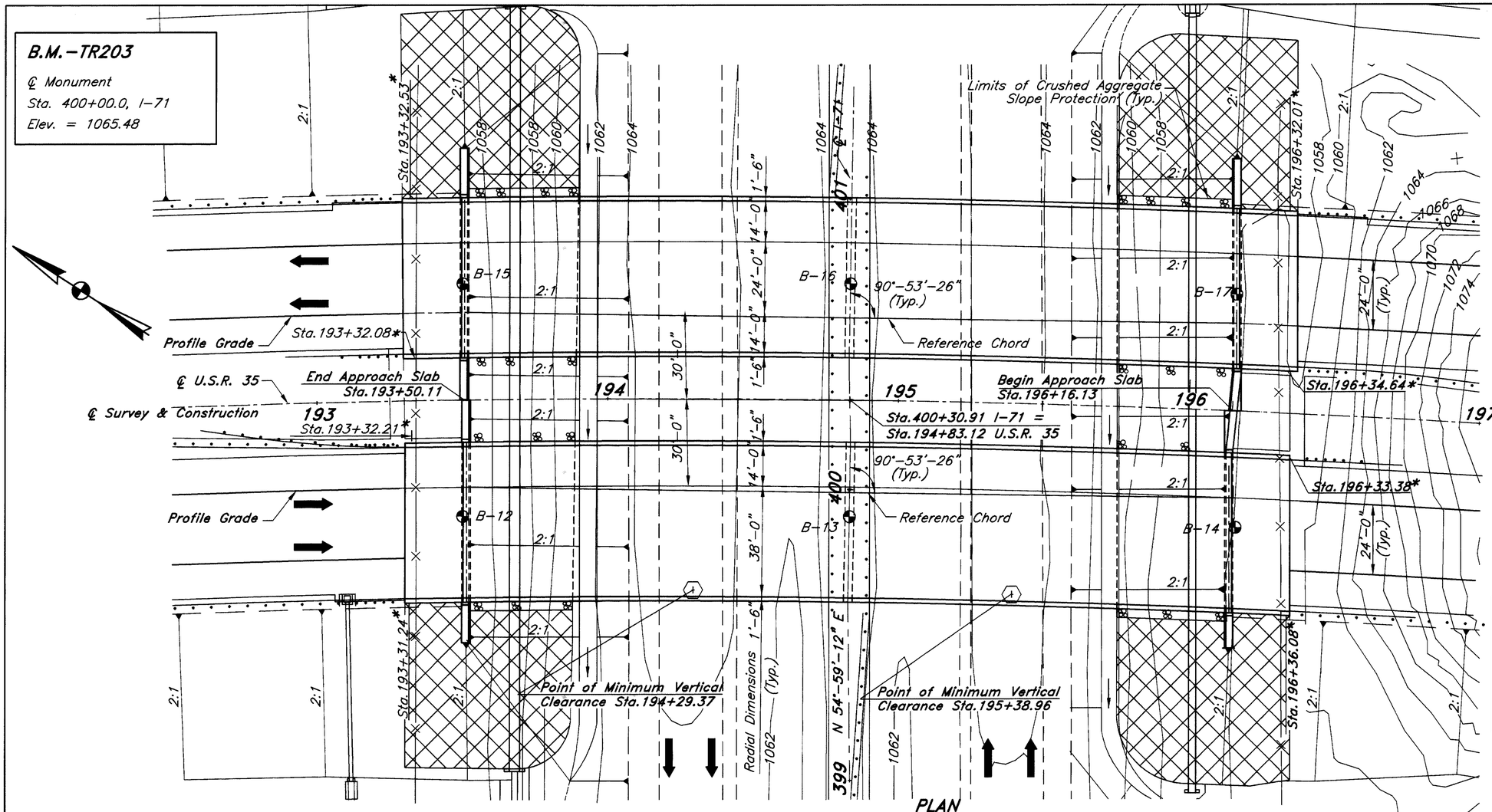
FAY-35-2.57

13 / 13

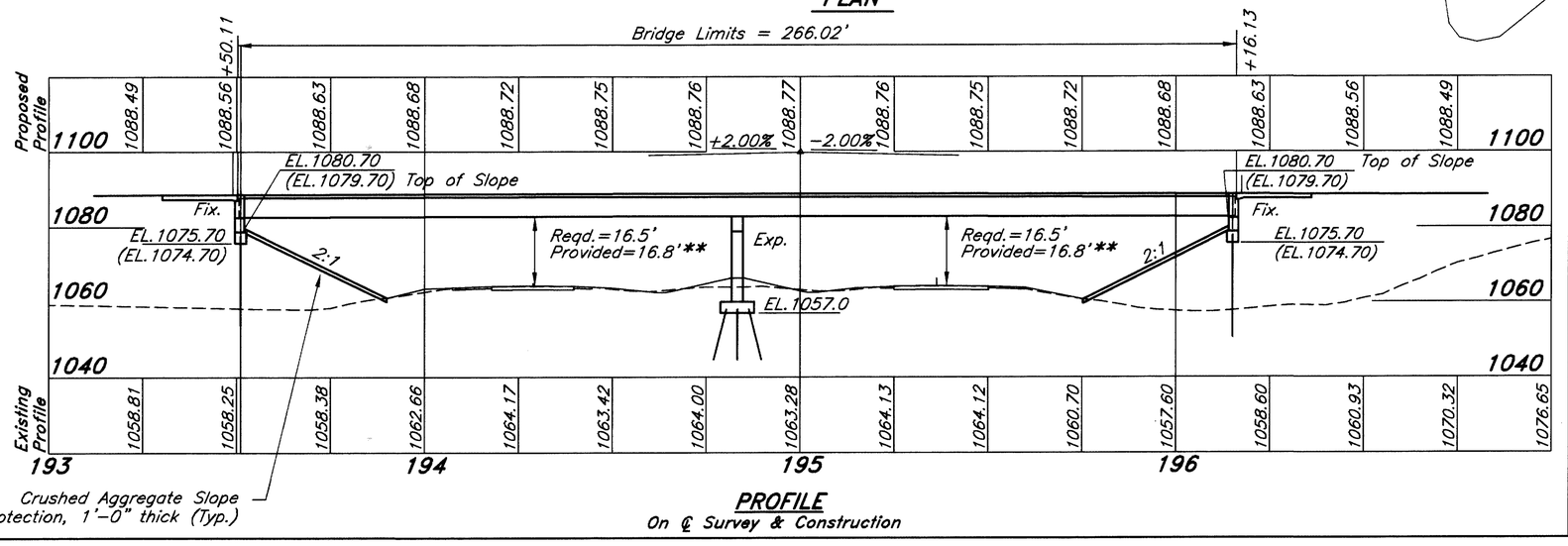
416
522

B.M.–TR203

℄ Monument
Sta. 400+00.0, I–71
Elev. = 1065.48



PLAN



PROFILE
On ℄ Survey & Construction

** After 6" of Scheduled Resurfacing on I–71

EL. in ( ) are for Right Bridge

EARTHWORK LIMITS shown are approximate. Actual slopes shall conform to plan cross–sections.

◒ B–12 thru B–17, Indicate Boring Locations.

DESIGN TRAFFIC

1997 ADT 1420
2017 ADT 1990
18% Trucks

HORIZONTAL CURVE DATA

P.I. Sta 196+74.95
Δ_c = 53°–16'–57" Rt.
D_c = 01°–15'–0"
R_c = 4583.66'
L_c = 4262.60'
T_c = 2299.46'
E = 544.44'

VERTICAL CURVE DATA

P.V.I. Sta. 195+00
P.V.I. EL. 1099.77
G_1 = +2.00%
G_2 = –2.00%
L = 2200'

PILE DATA

Location	Type	Estimated Pile Length
Abutments	12"ø Conc.	50'
Pier	12"ø Conc.	25'

* Stations are given at ℄ Guardrail Terminal Assembly.

PROPOSED STRUCTURE

TYPE : Continuous Plate Girders on Reinforced Conc. Substructure
SPAN: 2 Spans @ 132'–0" c/c Bearings Along ℄ Survey & Construction
ROADWAY: 52'–0" Toe/Toe Parapets
LOADING: HS20–44 (Case I) and the Alternate Military Loading
SKEW: 0°–53'–26" R.F. with Reference Chord
WEARING SURFACE: Monolithic Concrete
APPROACH SLABS: 20' Long (AS–1–81)
ALIGNMENT: Tangent
SUPERELEVATION : 0.044:1
LATITUDE : 39°–37'–25"N
LONGITUDE : 83°–36'–00"W
USGS QUADRANGLE : MILLEDGEVILLE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2–24–99
CHECKED	REVISED		
SP			

STRUCTURE FILE NO.2400359

FAYETTE COUNTY
STA.193+50.11
STA.196+16.13

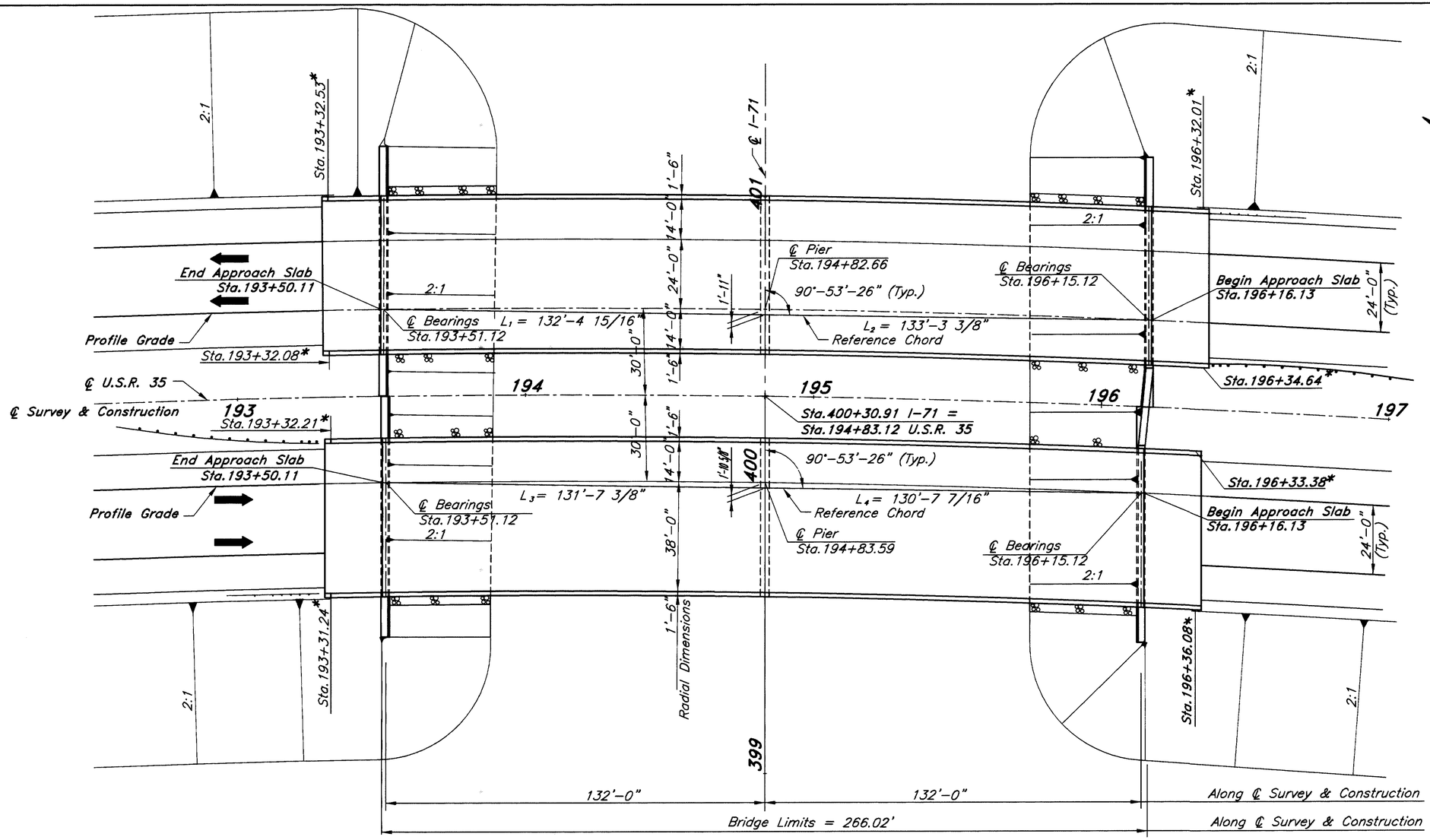
SITE PLAN
BRIDGE NO. FAY–35–0366 L & R.
U.S.35 OVER I–71

FAY–35–2.57

1 / 20

417 / 522

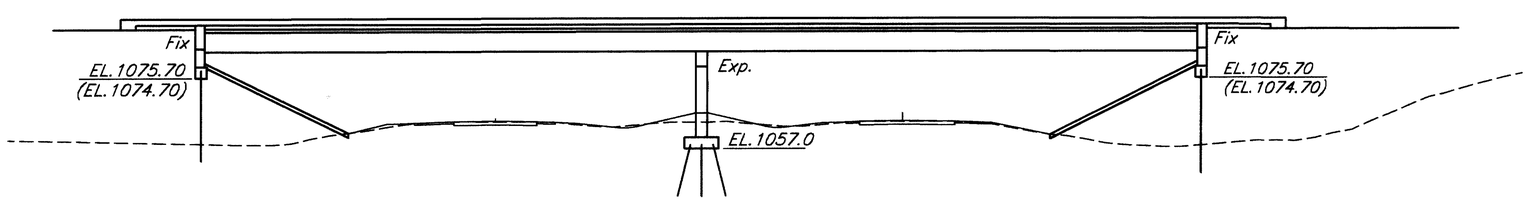
May 24, 1999 Q:\BRIDGE\Fay35\3595cspl.dwg



GENERAL PLAN

L_1, L_2, L_3 & L_4 are lengths along Reference Chord between ℄ Bearings.

* Stations are given at ℄ Guardrail Terminal Assembly



ELEVATION

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		STRUCTURE FILE NO. 2400359
SP			

GENERAL PLAN & ELEVATION
BRIDGE NO. FAY-35-0366 L. & R.
OVER I-71

FAY-35-2.57

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844	-	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C	-	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL	-	ASTM A615, A616 OR A617 GRADE 60 UNIT STRESS 24,000 PSI
STRUCTURAL STEEL	-	ASTM A572-YIELD STRENGTH 50,000 P.S.I.

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81 REV. 9-15-94 SHEET NO. 1. AND 2.
BR-1 DATED 1-6-99
GSD-1-96 DATED 2-12-97 SHEETS NO. 1 & NO. 3
ICD-1-82M DATED 3-20-95

AND TO SUPPLEMENTAL SPECIFICATIONS :

816 DATED 4-21-97
842 DATED 1-6-99
844 DATED 1-6-99
863 DATED 10-12-99

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, TOP & BOTTOM MATS 2 1/2" CONCRETE COVER TOP, 1 1/2" COVER BOTTOM AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED AND HAS UNDERGONE A WAITING PERIOD OF 30 DAYS.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 86 TONS PER PILE FOR THE 12" 0 CIP ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 96 TONS PER PILE FOR THE 12" CIP PIER PILES.

ABUTMENT PILES:
- 56 PILES 50 FEET LONG, ESTIMATED LENGTH
- 56 PILES OF ORDER LENGTH 55 FEET LONG
- 28 SPLICES

PIER PILES:
- 72 PILES 25 FEET LONG, ESTIMATED LENGTH
- 72 PILES OF ORDER LENGTH 30 FEET LONG
- 36 SPLICES

ABUTMENT BACKFILL

ABUTMENT BACKFILL SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

CONSTRUCTION CONSTRAINTS:

ALL EMBANKMENT MATERIAL FOR FILLING THE VOID CREATED BY EXCAVATION FOR THE ABUTMENT FOOTINGS SHALL BE 203 EMBANKMENT MATERIAL. AFTER THE FOOTING AND THE BREASTWALL HAVE BEEN CONSTRUCTED, THE VOID BEHIND EACH ABUTMENT SHALL BE FILLED UP TO THE BEAM SEAT ELEVATION AND FROM THE BEAM SEAT UP ON A 1:1 SLOPE TO THE SUBGRADE ELEVATION PRIOR TO CONSTRUCTING THE BACKWALL AND SETTING THE BEAMS ON THE ABUTMENT.

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.06 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

CONCRETE PARAPETS:

AS SOON AS A CONCRETE SAW CAN BE OPERATED WITHOUT DAMAGING THE FRESHLY PLACED CONCRETE, 1 INCH DEEP CONTROL JOINTS SHALL BE SAWED INTO THE PERIMETER OF THE CONCRETE PARAPET. THE SAW CUT SHALL BE MADE IN COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK. THE SAWCUTS SHALL BE PLACED AT A MINIMUM OF 6'-0" AND A MAXIMUM OF 10'-0" CENTERS. THE USE OF AN EDGE GUIDE FENCE, OR JIG IS REQUIRED TO INSURE THAT THE CUT JOINT IS STRAIGHT, TRUE, AND ALIGNED ON ALL FACES OF THE PARAPET. THE JOINT WIDTH SHALL BE THE WIDTH OF THE SAW BLADE, A NOMINAL WIDTH OF 1/4 INCH. THE PERIMETER OF THE DEFLECTION CONTROL JOINT SHALL BE SEALED TO A MINIMUM DEPTH OF 1 INCH WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION, TT-S-0027E TO A MINIMUM DEPTH OF 1 INCH. THE BOTTOM ONE HALF INCH OF BOTH THE INSIDE AND OUTSIDE FACES OF THE PARAPET SHOULD BE LEFT UNSEALED TO ALLOW ANY WATER WHICH MAY ENTER THE JOINT TO ESCAPE.

ALL LABOR, TOOLS, EQUIPMENT, MATERIALS AND INCIDENTALS NECESSARY TO COMPLETE THIS WORK ARE INCLUDED UNDER ITEM 842 CLASS S CONCRETE, SUPERSTRUCTURE.

ITEM 203 EMBANKMENT AS PER PLAN

ALL FILL MATERIAL FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT PLACED BETWEEN STATIONS 192+50 TO 193+00 AND FOR FILLING THE EXCAVATION VOID CREATED BY REMOVAL OF THE EXISTING ABUTMENTS, SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 5 /20 , 9 /20 , 10 / 20 AND 12 / 20
SEE PROPOSAL NOTE.

ITEM 503, UNCLASSIFIED EXCAVATION, AS PER PLAN

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN 6 INCH LIFTS.

May 24, 1999 C:\BRIDGE\Fay0R\3866sgn.dvg

DESIGN AGENCY
EMKSSON ENGINEERING LTD.
1333 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
VK			

STRUCTURE FILE NO.2400366

GENERAL NOTES
BRIDGE NO. FAY-35-0366
U.S.R.35 OVER I.R.71

FAY-35-2.57

3 / 20

419
522

ESTIMATED QUANTITIES

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	LEFT BRIDGE ABUT.	LEFT BRIDGE PIER	LEFT BRIDGE SUPER.	LEFT BRIDGE GEN.	RIGHT BRIDGE ABUT.	RIGHT BRIDGE PIER	RIGHT BRIDGE SUPER.
503	21101	580	CU YD	UNCLASSIFIED EXCAVATION, AS PER PLAN	170	125			170	125	
505	11100	LUMP	SUM	PILE DRIVING EQUIPMENT MOBILIZATION				LUMP			
507	00500	4600	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES DRIVEN	1400	900			1400	900	
507	00550	5240	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES FURNISHED	1540	1080			1540	1080	
507	50500	64	EACH	STEEL PILE SPLICES	14	18			14	18	
SPECIAL	51267500	1509	SQ YD	SEALING OF CONCRETE SURFACES *	83	144	533		83	140	526
516	13600	380	SQ FT	1" PREFORMED EXPANSION JOINT FILLER	190				190		
516	13200	228	SQ FT	1/2" PREFORMED EXPANSION JOINT FILLER	114				114		
516	44101	14	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.		7				7	
				20" x 28" x 2" NEOPRENE WITH 2" x 21" x 27" STEEL							
				LOAD PLATE, AS PER PLAN							
516	30501	321	LIN. FT.	PVC WATERSTOP, AS PER PLAN	161				161		
518	21200	234	CU YD	POROUS BACKFILL WITH FILTER FABRIC	117				117		
518	40000	330	LIN FT	6" PERFORATED CORRUGATED PLASTIC PIPE	165				165		
518	40010	114	LIN FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE,	57				57		
				INCLUDING SPECIALS							
816	00600	LUMP	SUM	PAINTING OF NEW STEEL, INTERMEDIATE AND FINISH COAT,			LUMP				LUMP
				SYSTEM IZEU							
844	48000	1028	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (DECK)			514				514
844	48020	158	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (PARAPET)			79				79
844	49000		LUMP	HIGH PERFORMANCE CONCRETE, TRIAL MIX							
844	49010		LUMP	HIGH PERFORMANCE CONCRETE, TESTING							
842	41000	87	CU YD	CLASS C CONCRETE, PIER CAPS AND COLUMNS		44				43	
842	43500	260	CU YD	CLASS C CONCRETE, ABUTMENT INCLUDING FOOTING	130				130		
842	46500	124	CU YD	CLASS C CONCRETE, FOOTING		62				62	
863	10060	LUMP	SUM	STRUCTURAL STEEL, MEMBERS LEVEL FOUR (4) FABRICATION			LUMP				LUMP
863	20000	5586	EACH	WELDED STUD SHEAR CONNECTORS			2793				2793

Computed by: S.P. Date: 10-12-94

Checked by: V.K. Date: 10-20-94

* SEE PROPOSAL NOTE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
VK			

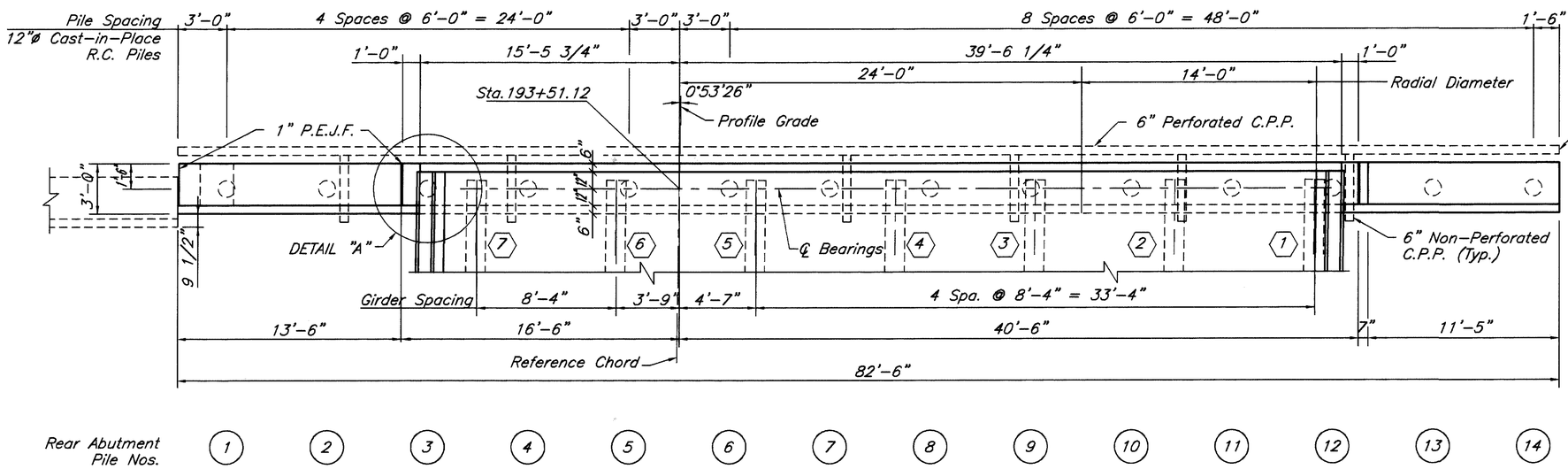
STRUCTURE FILE NO. 2400369

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0366
U.S.R.35 OVER I.R.71

FAY-35-2.57

Revised 8/11/00 by E.R.P.

May 24, 1999 O:\BRIDGE\Fay35\366qnty.dwg



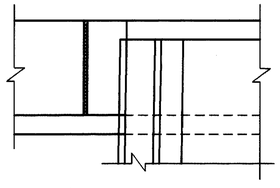
Rear Abutment

Numbers in (1) are Girder Nos.

PLAN – REAR ABUTMENT
LEFT BRIDGE

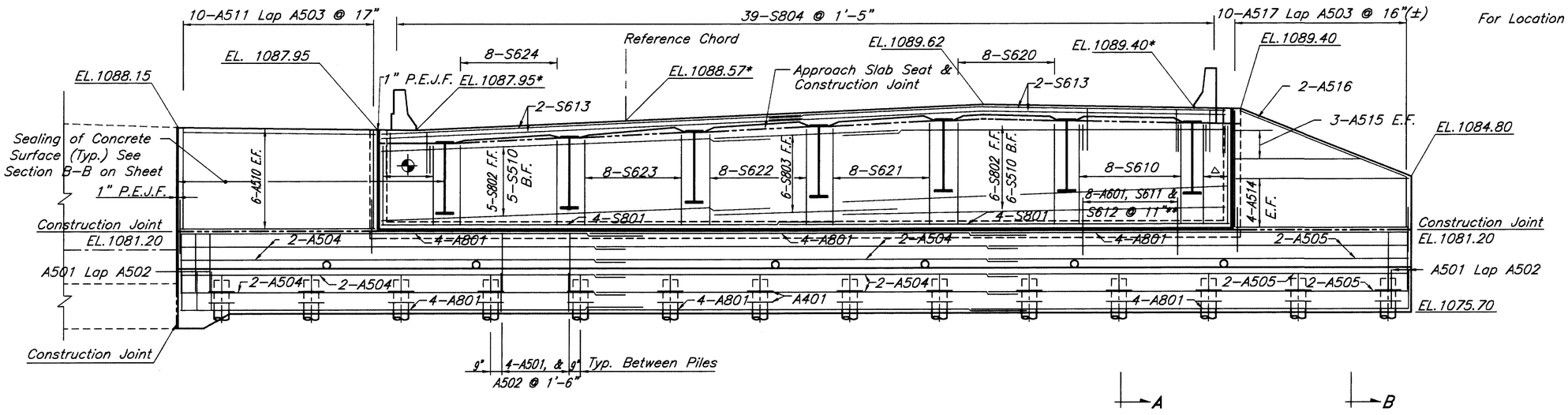
LEFT BRIDGE		
Girder Nos.	Dim "A" Rear Abut.	Dim "A" Fwd. Abut.
1	2'-5 3/8"	2'-6 3/8"
2	2'-6 15/16"	2'-8 1/16"
3	2'-6 5/8"	2'-7 5/8"
4	2'-2 1/4"	2'-3 3/16"
5	1'-9 13/16"	1'-10 13/16"
6	1'-5 7/16"	1'-6 7/16"
7	1'-1 1/16"	1'-2"

DETAIL "A"



A

B



A

B

For Location of Dim "A" See Sheet 9|20

* Elevations are given @ ℄ Bearings

For notes, and Sections A–A & B–B see sheet 9|20

** Typical Between Girders

◈ = 4–A601, S624, S611 and S612

△ = 2–A601, S610, S611 and S612

ELEVATION

Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Plastic Pipe

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED

STRUCTURE FILE NO.2400359

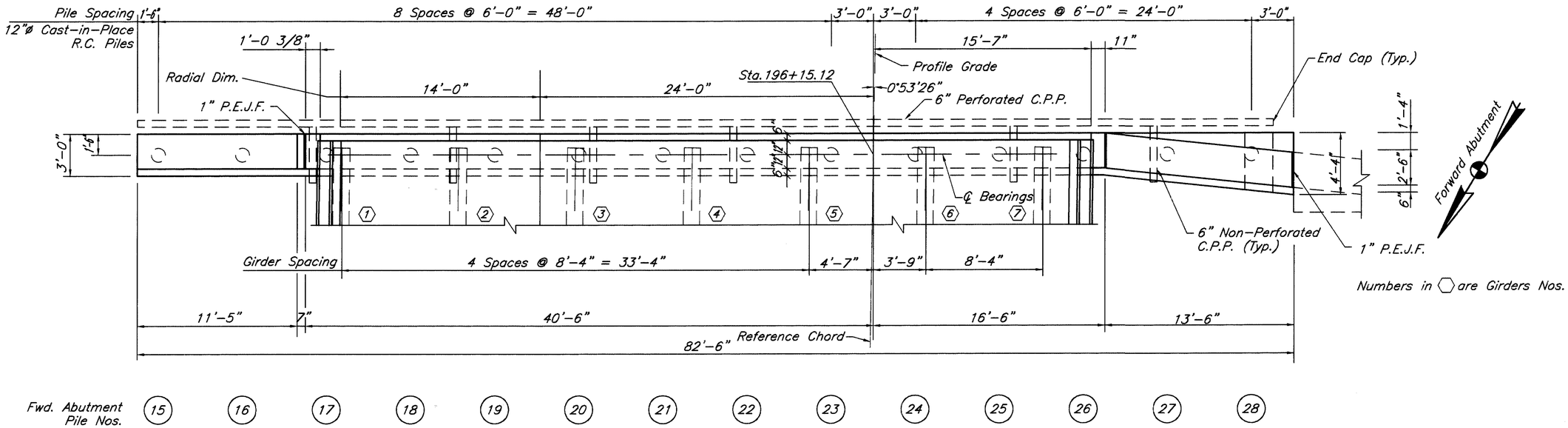
REAR ABUTMENT DETAILS
Bridge No. FAY-35-0366 (LEFT)
US 35 OVER I-71

FAY-35-2.57

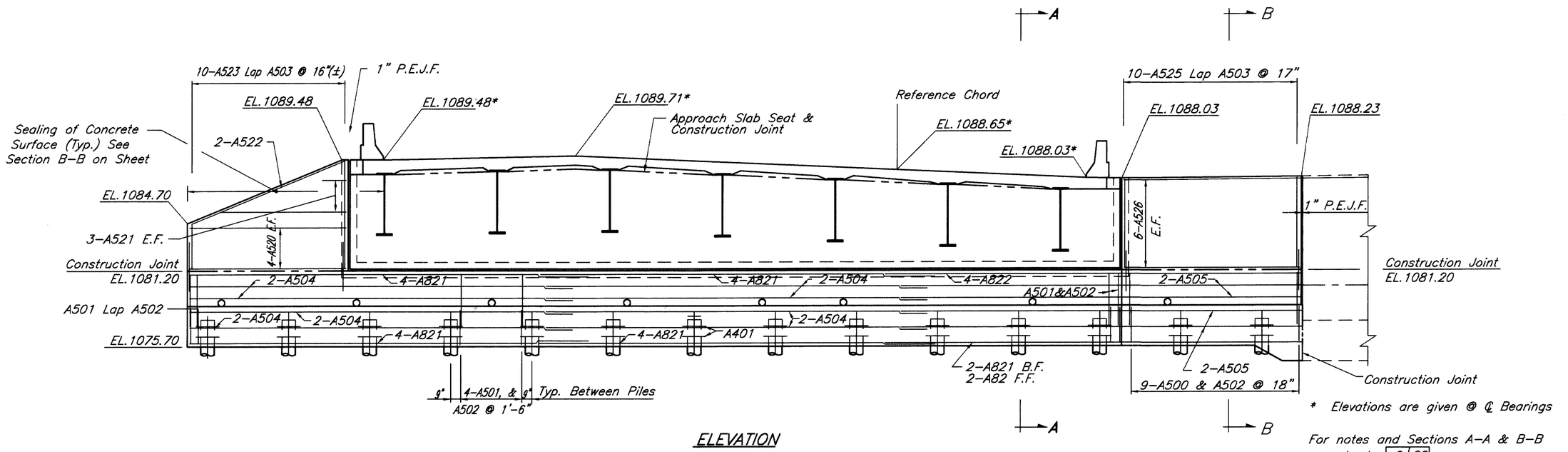
5 | 20

421
522

May 24, 1999 Q:\BRIDGE\Fay35\3595cab1



PLAN – FORWARD ABUTMENT
LEFT BRIDGE



ELEVATION

* Elevations are given @ ℄ Bearings

For notes and Sections A–A & B–B see sheet 9 | 20

ABBREVIATIONS:

C.P.P. = Corrugated Plastic Pipe

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

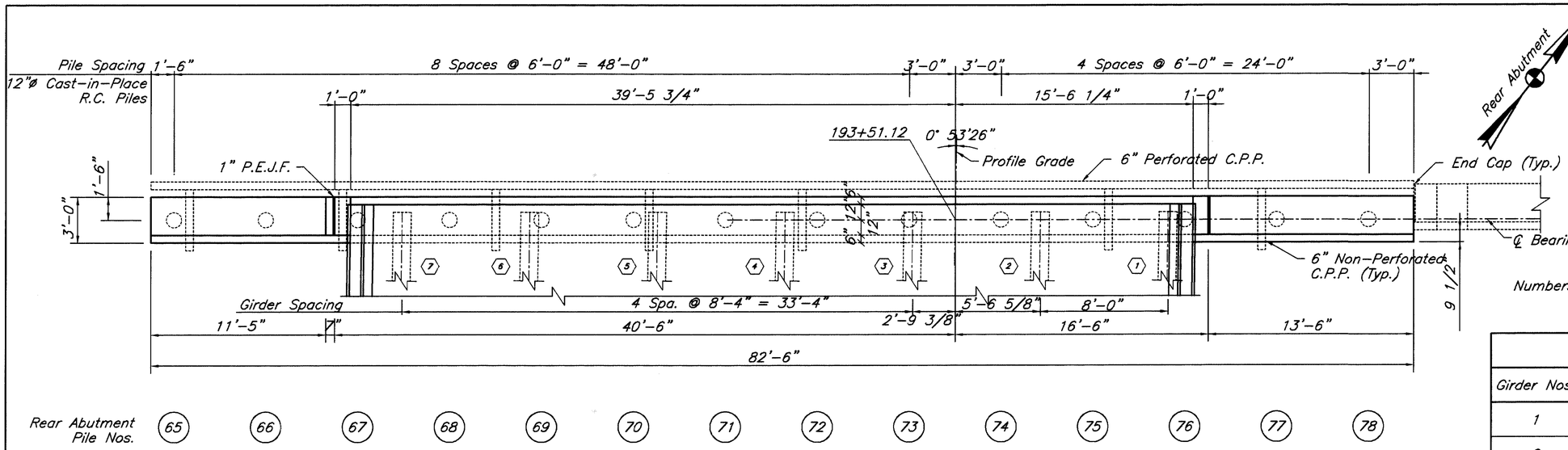
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400359

FORWARD ABUTMENT DETAILS
Bridge No. FAY-35-0366 (LEFT BRIDGE)
US 35 OVER I-71

FAY-35-2.57

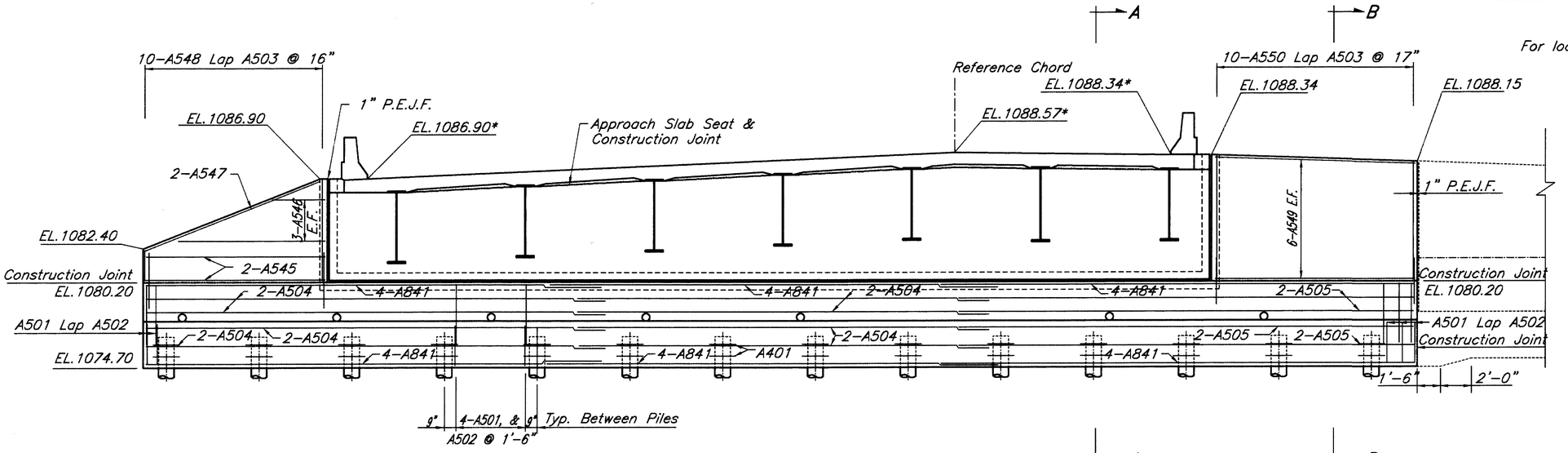
May 24, 1999 Q:\BRIDGE\Fay35\3595cab2.dwg



PLAN – REAR ABUTMENT
RIGHT BRIDGE

RIGHT BRIDGE		
Girder Nos.	Dim "A" Rear Abut.	Dim "A" Fwd. Abut.
1	2'-4 11/16"	2'-5 5/8"
2	2'-6 1/4"	2'-7 1/4"
3	2'-5 15/16"	2'-6 15/16"
4	2'-1 9/16"	2'-2 1/2"
5	1'-9 1/8"	1'-10 1/8"
6	1'-4 3/4"	1'-5 11/16"
7	1'-0 5/16"	1'-1 5/16"

For location of Dim. "A" See Sheet 9|20



ABBREVIATIONS

C.P.P. = Corrugated Plastic Pipe

* Elevations are given @ ℄ Bearings

For notes and Sections A–A & B–B see sheet 9|20

ELEVATION

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.2400359

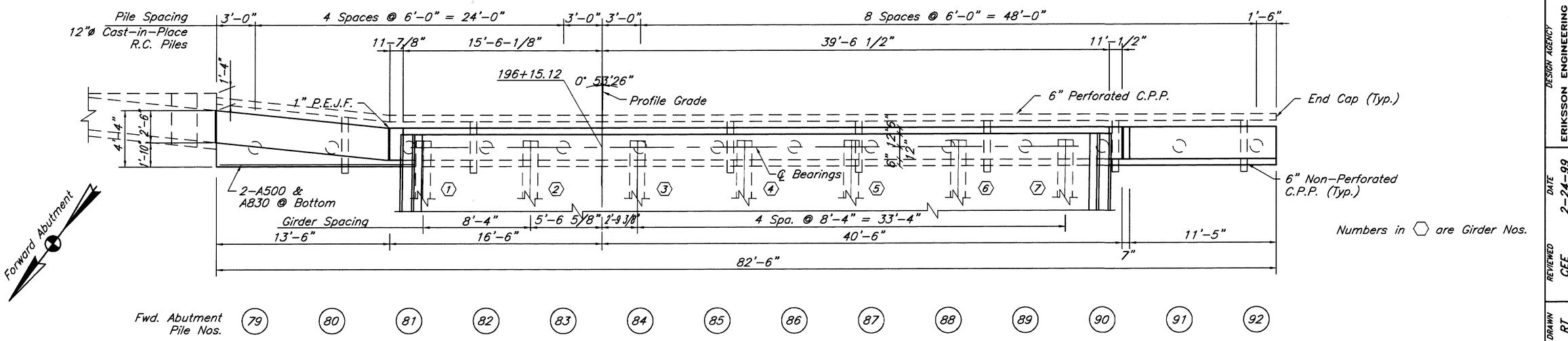
REAR ABUTMENT DETAILS
Bridge No. FAY-35-0366 (RIGHT)
US 35 OVER I-71

FAY-35-2.57

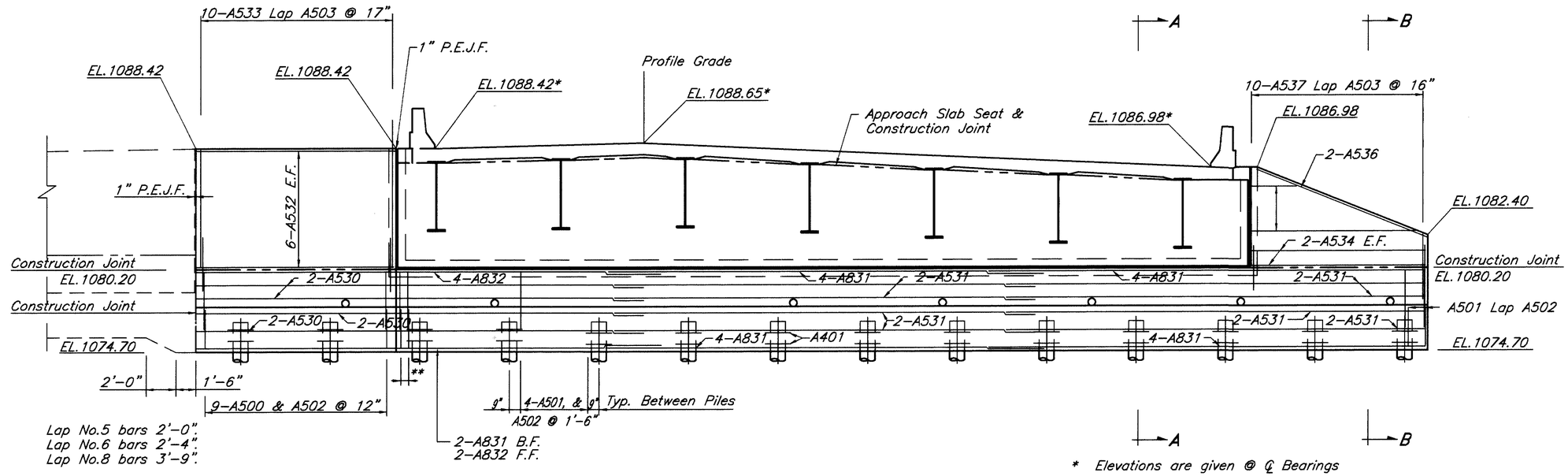
7 | 20

423
522

May 24, 1999 Q:\BRIDGE\Fay35\3595cab4.dwg



PLAN – FORWARD ABUTMENT
RIGHT BRIDGE



Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

ABBREVIATIONS
F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Plastic Pipe

** A501 & A502

ELEVATION

* Elevations are given @ ℄ Bearings

For notes and Sections A–A & B–B see sheet 9|20

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO. 2400359

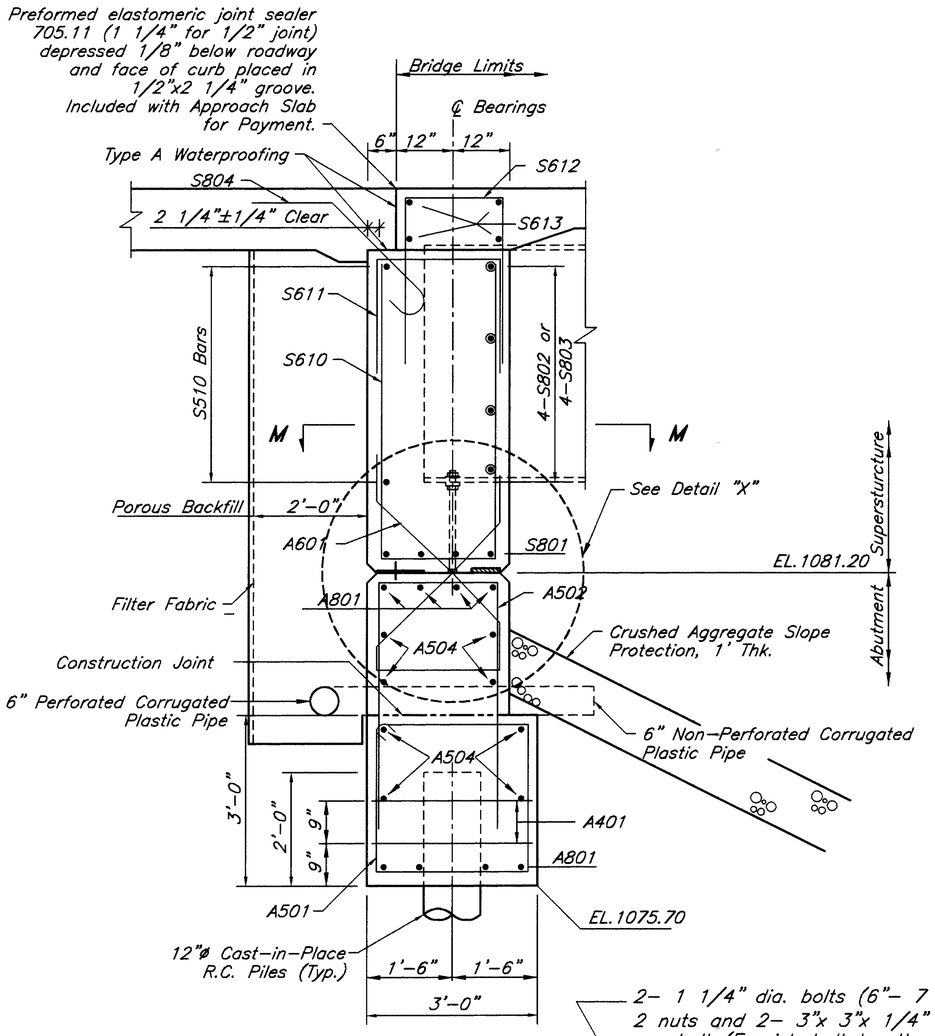
FORWARD ABUTMENT DETAILS
Bridge No. FAY-35-0366 (RIGHT)
US 35 OVER I-71

FAY-35-2.57

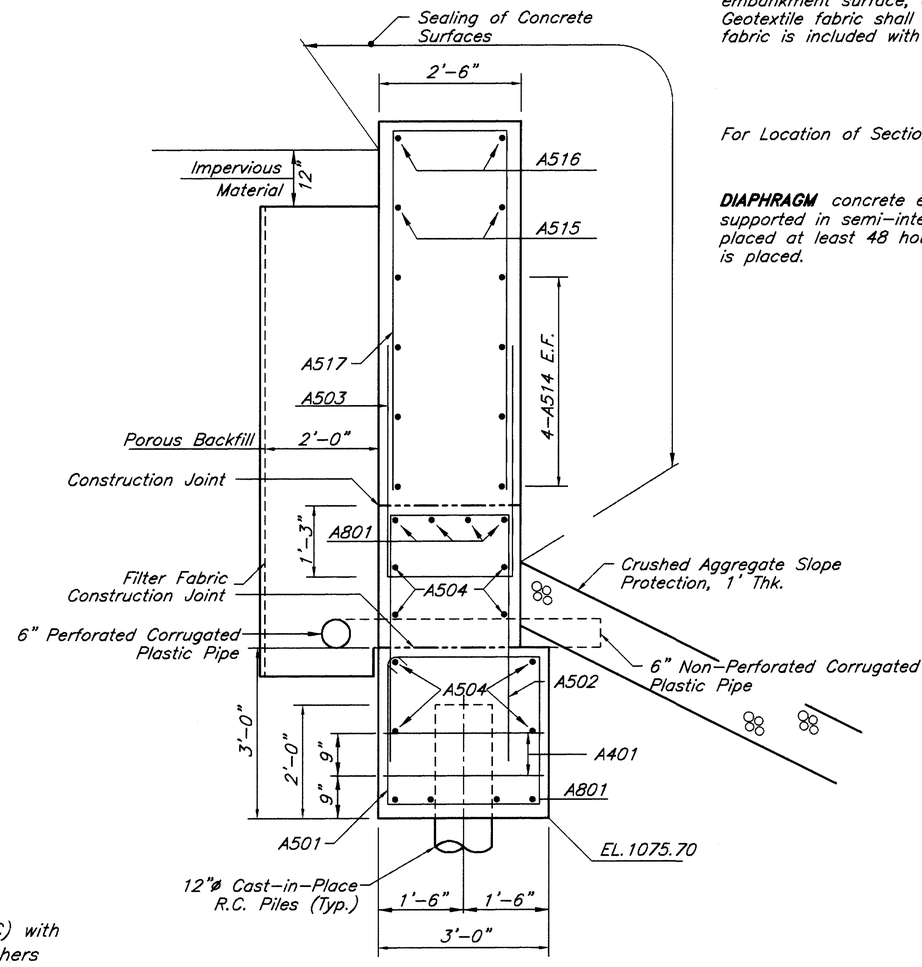
8 | 20

424
522

May 24, 1999 Q:\BRIDGE\Fay35\3595cab5.dwg



SECTION A-A

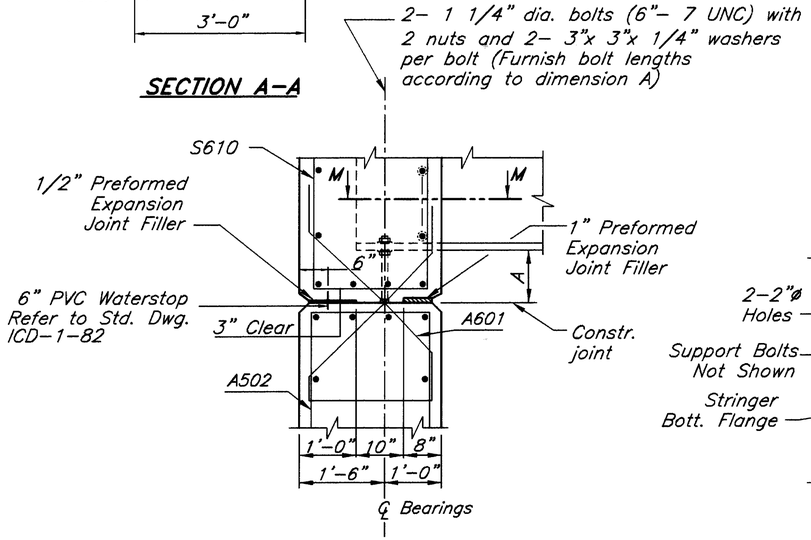


SECTION B-B

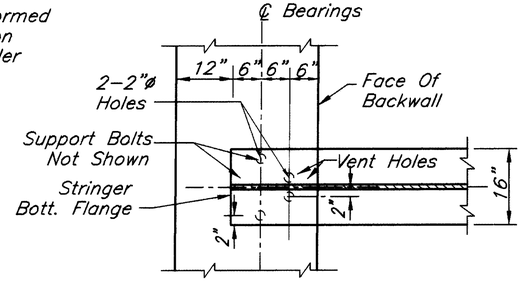
POROUS BACKFILL *with filter fabric shall extend upward to the plane of the subgrade, to one foot below the embankment surface, and laterally to the ends of the wingwalls. Geotextile fabric shall conform with 712.09, Type A. Geotextile fabric is included with porous backfill for payment.*

For Location of Sections A-A & B-B see sheet 5/20

DIAPHRAGM *concrete encasing the structural member sections supported in semi-integral and integral type abutments shall be placed at least 48 hours before the actual deck concrete is placed.*



DETAIL X



SECTION M-M

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVIEWED CEE
DATE 2-24-99
REVISED
STRUCTURE FILE NO.2400359

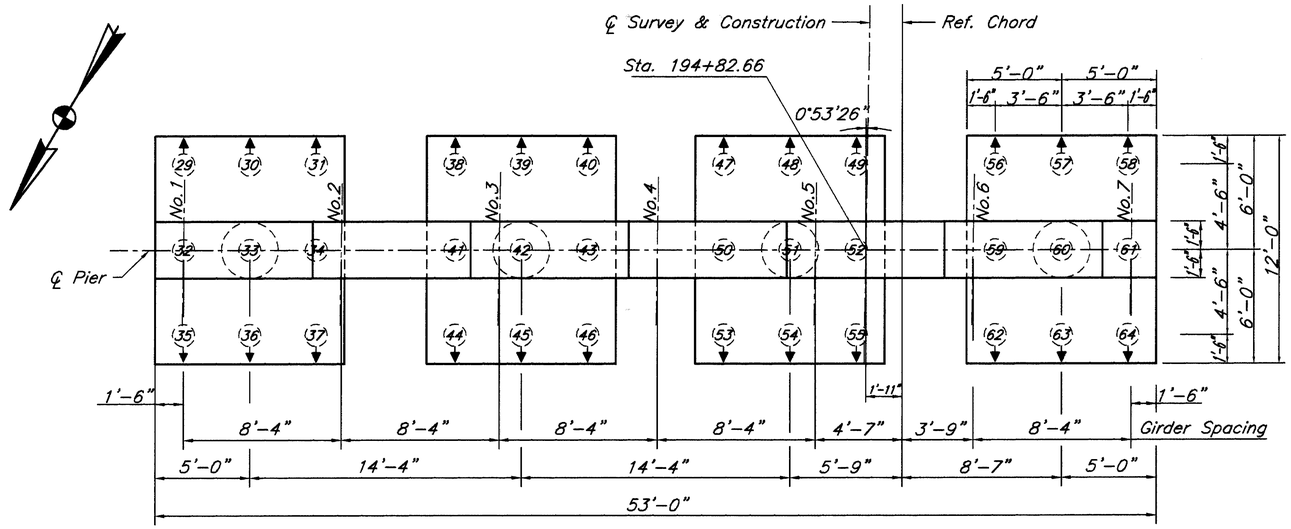
ABUTMENT DETAILS
Bridge No. FAY-35-0366
US 35 OVER I-71

FAY-35-2.57

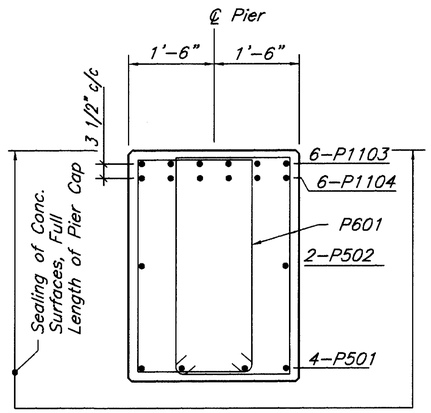
9/20

425
522

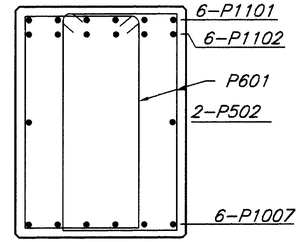
May 24, 1999 Q:\BRIDGE\Fay35\3595cab3.dwg



PLAN
LEFT BRIDGE, PIER 1



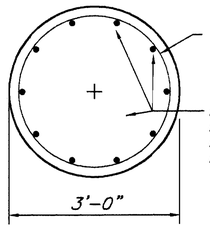
SECTION A-A



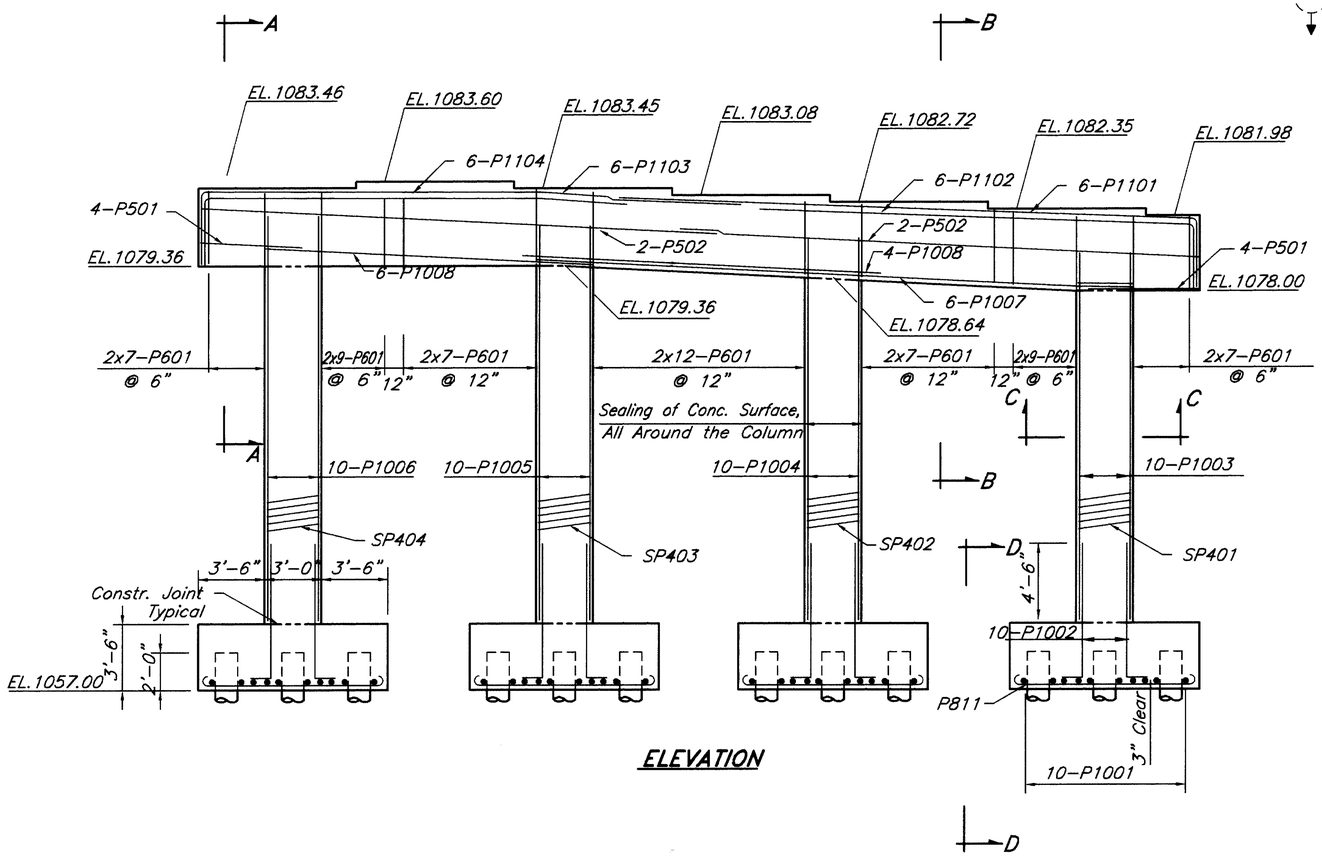
SECTION B-B

NOTE: Footing & Pile Dimensions are Typical for All Footings.

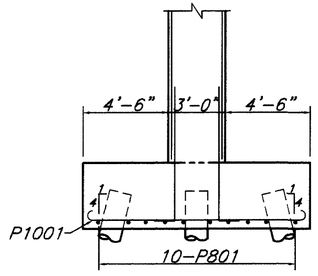
Indicates Piles Battered at 1:4



SECTION C-C



ELEVATION



VIEW D-D

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

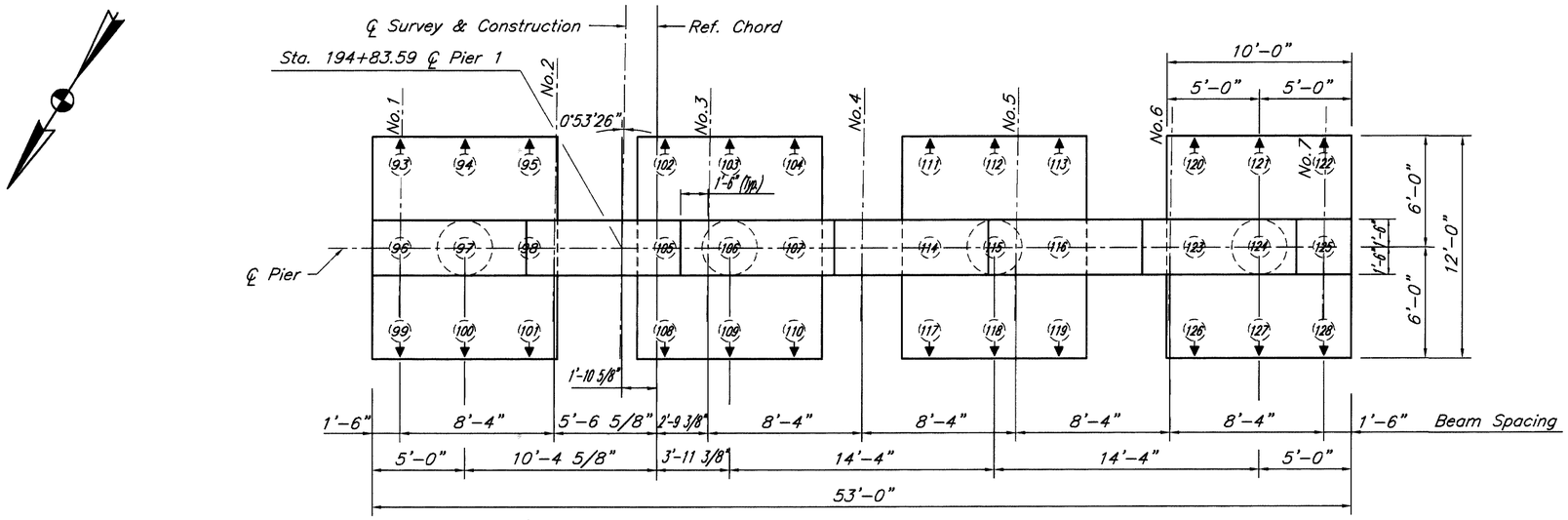
DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO.2400359
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

LEFT BRIDGE PIER DETAILS
Bridge No. FAY-35-0366 (LEFT)
U.S.35 OVER I-71

FAY-35-2.57

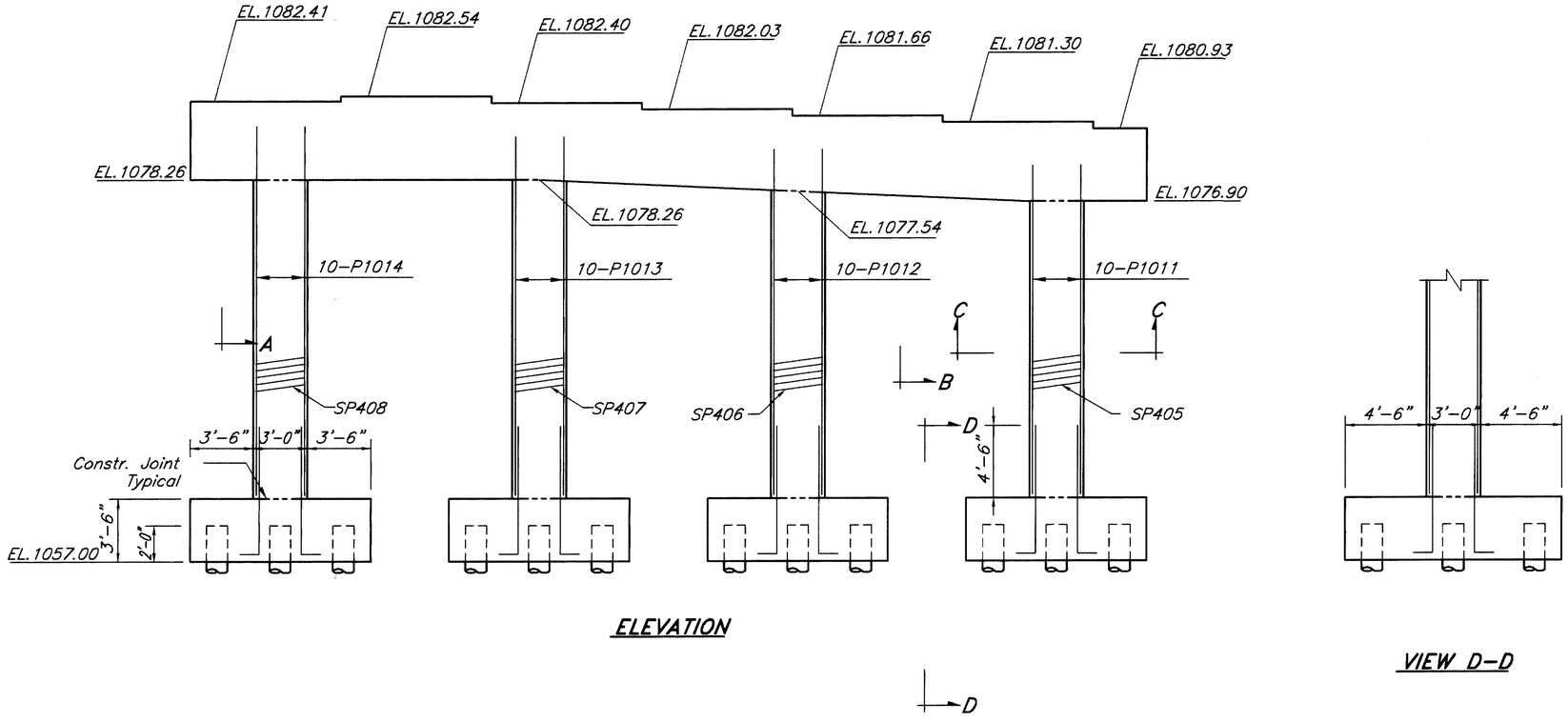
10 / 20

426
522



PLAN

RIGHT BRIDGE, PIER 1



ELEVATION

VIEW D–D

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

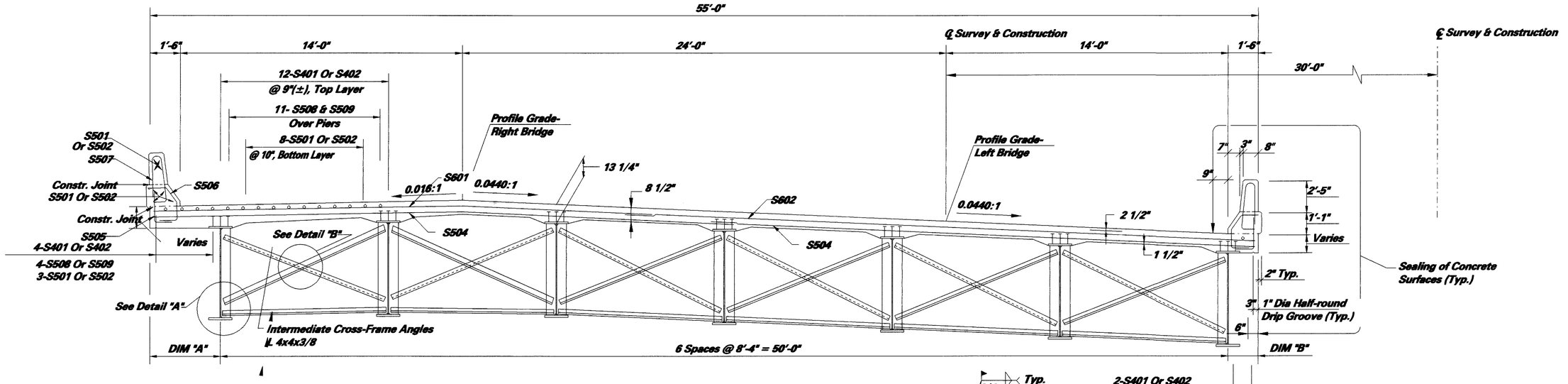
DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVISED
REVIEWED CEE
DATE 2-24-99
STRUCTURE FILE NO.2400359

PIER DETAILS
Bridge No. FAY-35-0366 (RIGHT)
U.S.35 OVER I-71

FAY-35-2.57

11 / 20

427
522



TRANSVERSE SECTION

A = 11-S505, S506 & S507 @ 1'-4"

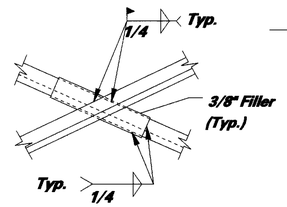
B = 6-S505, S506 & S507 @ 1'-5"

	AT REAR ABUT. BRG.	℄ PIER	AT FWD. ABUT. BRG.
DIM "A"	1'-7"	3'-6"	1'-7 1/2"
DIM "B"	3'-5"	1'-6"	3'-5 1/8"

Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

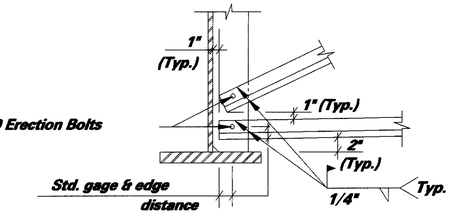
** 71-S508 and 71-S509 required over pier

For Notes See Sheet 13/20

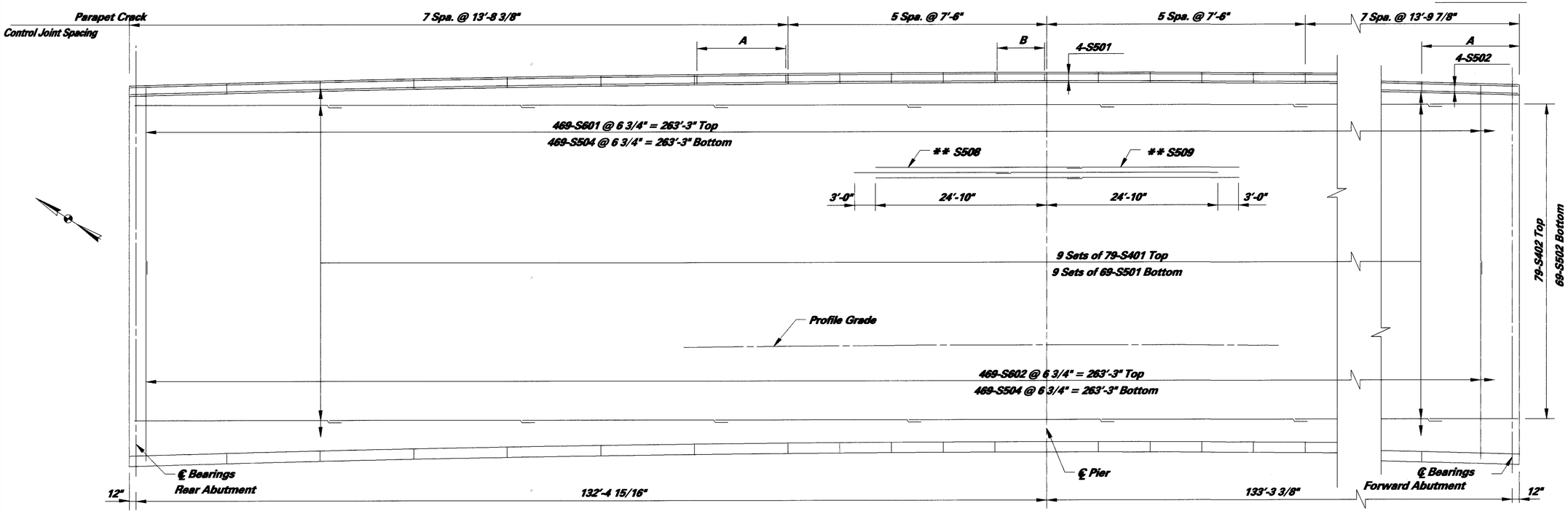


DETAIL "B"

2-S401 Or S402
1-S508 Or S509
2-S501 Or S502



DETAIL A



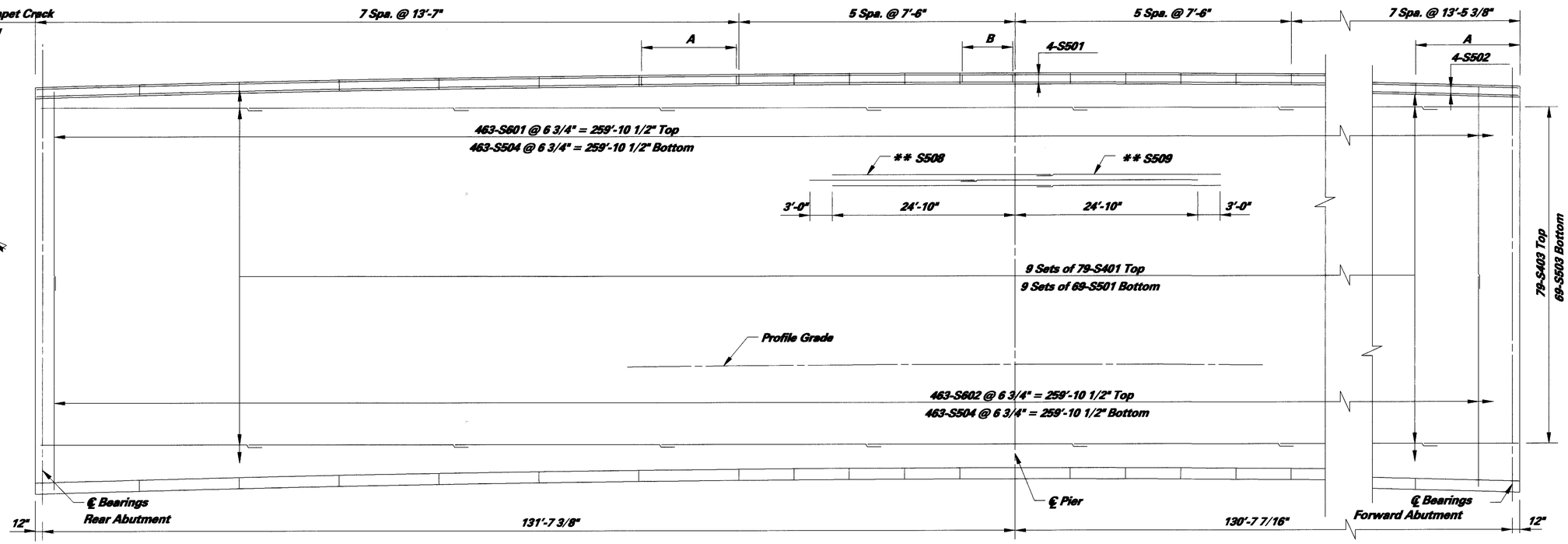
DECK REINFORCING PLAN-LEFT BRIDGE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-09
REVIEWED CEE
STRUCTURE FILE NO. 3400389
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0366 (LEFT)
US 35 OVER I-71

FAY-35-2.57



DECK REINFORCING PLAN-RIGHT BRIDGE

DIM "A" and DIM "B" See Sheet 12/20

	AT REAR ABUT. BRG.	℄ PIER	AT FWD. ABUT. BRG.
DIM "A"	1'-7 3/8"	3'-6"	1'-7 1/2"
DIM "B"	3'-4 5/8"	1'-6"	3'-5 1/8"

NOTES:

THE DISTANCE SHOWN FROM THE TOP OF THE DECK SLAB TO THE BOTTOM OF THE TOP FLANGE IS THE THEORETICAL DESIGN DIMENSION. THE QUANTITY OF DECK CONCRETE TO BE PAID FOR SHALL BE BASED UPON THIS DIMENSION, EVEN THOUGH DEVIATION FROM IT MAY BE NECESSARY BECAUSE THE TOP FLANGE OF THE GIRDER MAY NOT HAVE THE EXACT CAMBER OR CONFORMATION REQUIRED TO PLACE IT PARALLEL TO THE FINISHED GRADE. DEDUCTION SHALL BE MADE FOR VOLUME OF ENCASED STEEL PLATES AS PER 842.

CONCRETE DECK HAUNCH WIDTH

A HAUNCH WIDTH OF 9 INCHES SHALL BE USED FOR COMPUTING QUANTITY OF CONCRETE. HOWEVER, THE HAUNCH WIDTH MAY VARY BETWEEN 6 AND 12 INCHES.

ERECTION BOLTS: THE HOLE DIAMETER IN THE CROSS FRAMES AND GIRDER STIFFENERS SHALL BE 3/16" LARGER THAN THE DIAMETER OF THE ERECTION BOLTS. UNLESS REPLACED BY PERMANENT HIGH STRENGTH BOLTS, ERECTION BOLTS SHALL REMAIN IN PLACE. LOCK WASHERS SHALL BE FURNISHED FOR OTHER THAN FULLY TORQUED HIGH STRENGTH ERECTION BOLTS. BOLTS SHALL BE FURNISHED AS PART OF ITEM 863.

IN LIEU OF ERECTION BOLTS AND AT THE OPTION OF THE CONTRACTOR, ALTERNATIVE MEANS OF TEMPORARY BRACING MAY BE USED SUBJECT TO THE APPROVAL OF THE DIRECTOR (501.06).

A = 11-S505, S506 & S507 @ 1'-4"

B = 6-S505, S506 & S507 @ 1'-5"

Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

*** 71-S508 and 71-S509 required over pier*

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400389

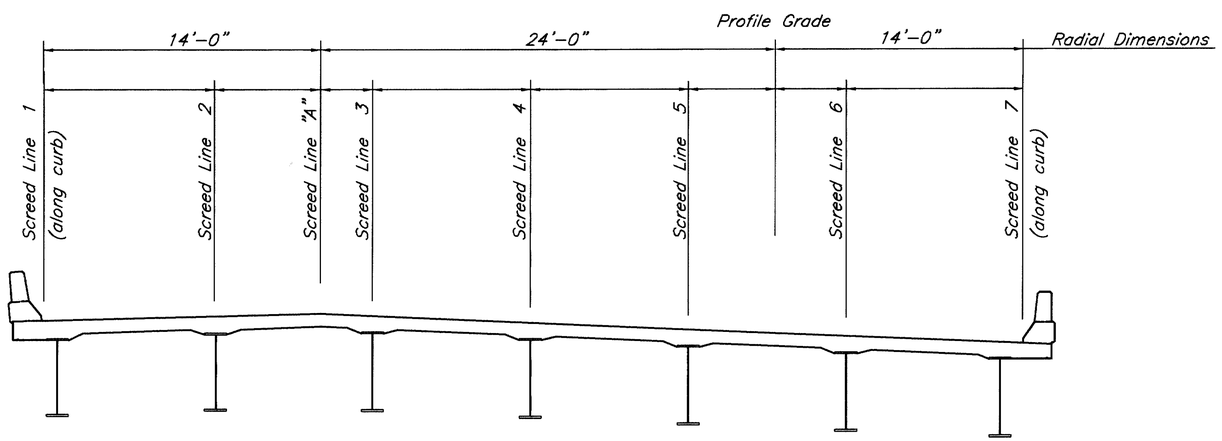
SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0368 (RIGHT)
US 35 OVER I-71

FAY-35-2.57

May 24, 1999 C:\BRIDGE\Fay35\3586ctk1.dwg

SCREED ELEVATION TABLE – LEFT BRIDGE

		Screed Line "A"		BEAM 1 (along curb)		BEAM 2		BEAM 3		BEAM 4		BEAM 5		BEAM 6		BEAM 7 (along curb)	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	193+50.75	1089.62	193+50.53	1089.40	193+50.66	1089.53	193+50.79	1089.50	193+50.92	1089.14	193+51.05	1088.77	193+51.18	1088.40	193+51.31	1087.95
	0.10 L	193+63.95	1089.71	193+63.73	1089.49	193+63.86	1089.63	193+63.99	1089.57	193+64.12	1089.21	193+64.25	1088.84	193+64.38	1088.47	193+64.51	1088.04
	0.20 L	193+77.15	1089.79	193+76.93	1089.56	193+77.06	1089.61	193+77.19	1089.63	193+77.32	1089.26	193+77.45	1088.90	193+77.58	1088.53	193+77.71	1088.11
	0.30 L	193+90.35	1089.84	193+90.13	1089.62	193+90.26	1089.77	193+90.39	1089.68	193+90.52	1089.31	193+90.65	1088.94	193+90.78	1088.58	193+90.91	1088.17
	0.40 L	194+03.55	1089.88	194+03.33	1089.65	194+03.46	1089.81	194+03.59	1089.70	194+03.72	1089.34	194+03.85	1088.97	194+03.98	1088.60	194+04.11	1088.20
	0.50 L	194+16.75	1089.89	194+16.53	1089.66	194+16.66	1089.70	194+16.79	1089.71	194+16.92	1089.34	194+17.05	1088.97	194+17.18	1088.61	194+17.31	1088.22
	0.60 L	194+29.95	1089.88	194+29.73	1089.66	194+29.86	1089.82	194+29.99	1089.69	194+30.12	1089.33	194+30.25	1088.96	194+30.38	1088.59	194+30.51	1088.21
	0.70 L	194+43.15	1089.87	194+42.93	1089.64	194+43.06	1089.80	194+43.19	1089.67	194+43.32	1089.30	194+43.45	1088.94	194+43.58	1088.57	194+43.71	1088.19
	0.80 L	194+56.35	1089.85	194+56.13	1089.62	194+56.26	1089.79	194+56.39	1089.64	194+56.52	1089.28	194+56.65	1088.91	194+56.78	1088.54	194+56.91	1088.17
	0.90 L	194+69.55	1089.83	194+69.33	1089.60	194+69.46	1089.77	194+69.59	1089.62	194+69.72	1089.26	194+69.85	1088.89	194+69.98	1088.52	194+70.11	1088.16
Span No.2	0.00 L	194+82.75	1089.82	194+82.53	1089.60	194+82.66	1089.76	194+82.79	1089.62	194+82.92	1089.25	194+83.05	1088.88	194+83.18	1088.52	194+83.31	1088.15
	0.10 L	194+95.95	1089.84	194+95.73	1089.61	194+95.86	1089.78	194+95.99	1089.63	194+96.12	1089.27	194+96.25	1088.90	194+96.38	1088.53	194+96.51	1088.16
	0.20 L	195+0915	1089.86	195+08.93	1089.64	195+09.06	1089.80	195+09.19	1089.66	195+09.32	1089.29	195+09.45	1088.93	195+09.58	1088.56	195+09.71	1088.19
	0.30 L	195+22.35	1089.89	195+22.13	1089.67	195+22.26	1089.83	195+22.39	1089.69	195+22.52	1089.33	195+22.65	1088.96	195+22.78	1088.60	195+22.91	1088.22
	0.40 L	195+35.55	1089.92	195+35.33	1089.69	195+35.46	1089.85	195+35.59	1089.72	195+35.72	1089.36	195+35.85	1088.99	195+35.98	1088.62	195+36.11	1088.24
	0.50 L	195+48.75	1089.9304	195+48.53	1089.71	195+48.66	1089.86	195+48.79	1089.75	195+48.92	1089.38	195+49.05	1089.01	195+49.18	1088.64	195+49.31	1088.26
	0.60 L	195+61.95	1089.93	195+61.73	1089.70	195+61.86	1089.86	195+61.99	1089.75	195+62.12	1089.38	195+62.25	1089.02	195+62.38	1088.65	195+62.51	1088.25
	0.70 L	195+75.15	1089.91	195+74.93	1089.68	195+75.06	1089.83	195+75.19	1089.73	195+75.32	1089.37	195+75.45	1089.00	195+75.58	1088.63	195+75.71	1088.23
	0.80 L	195+88.35	1089.85	195+88.13	1089.63	195+88.26	1089.77	195+88.39	1089.70	195+88.52	1089.33	195+88.65	1088.97	195+88.78	1088.60	195+88.91	1088.18
	0.90 L	196+01.55	1089.78	196+01.33	1089.56	196+01.46	1089.70	196+01.59	1089.65	196+01.72	1089.28	196+01.85	1088.91	196+01.98	1088.55	196+02.11	1088.11
	1.00 L	196+14.75	1089.71	196+14.53	1089.48	196+14.66	1089.62	196+14.79	1089.58	196+14.92	1089.22	196+15.05	1088.85	196+15.18	1088.48	196+15.31	1088.03



SCREED LINE LOCATIONS – LEFT BRIDGE

SCREED ELEVATIONS *are for the concrete surface before the concrete is placed. Allowance has been made for the anticipated deflection of the structural steel.*

L = Span Length

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVIEWED CEE
DATE 2-24-99
REVISED
STRUCTURE FILE NO.2400359

SCREED ELEVATIONS
Bridge No. FAY-35-0366 (LEFT)
U.S.35 OVER I-71

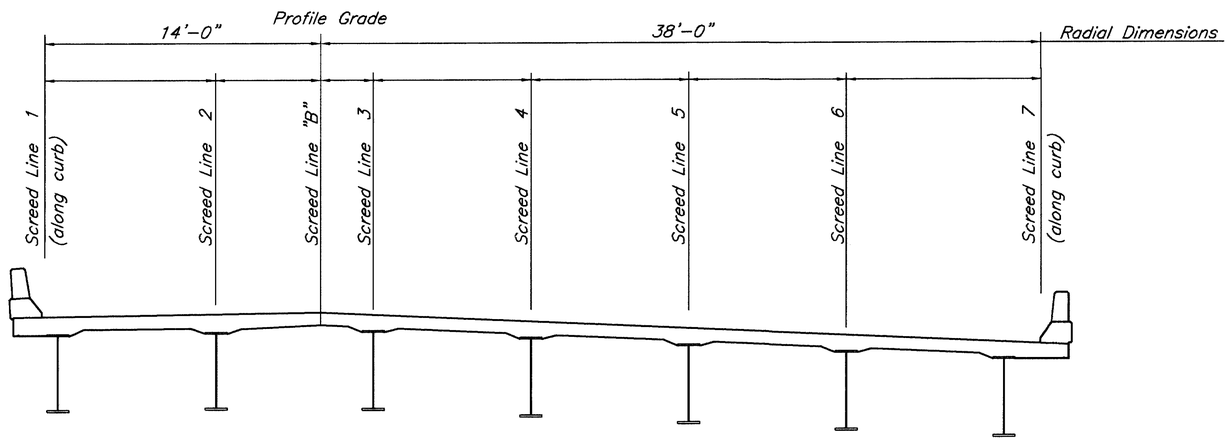
FAY-35-2.57

SCREED ELEVATION TABLE – RIGHT BRIDGE

		SCREED LINE "B"		BEAM 1 (along curb)		BEAM 2		BEAM 3		BEAM 4		BEAM 5		BEAM 6		BEAM 7 (along curb)	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	193+51.12	1088.57	193+50.90	1088.34	193+51.03	1088.48	193+51.16	1088.45	193+51.29	1088.08	193+51.42	1087.71	193+51.55	1087.35	193+51.68	1086.90
	0.10 L	193+64.32	1088.66	193+64.10	1088.43	193+64.23	1088.57	193+64.36	1088.52	193+64.49	1088.15	193+64.62	1087.78	193+64.75	1087.42	193+64.88	1086.98
	0.20 L	193+77.52	1088.73	193+77.30	1088.59	193+77.43	1088.65	193+77.56	1088.68	193+77.69	1088.21	193+77.82	1087.85	193+77.95	1087.48	193+78.08	1087.06
	0.30 L	193+90.72	1088.79	193+90.50	1088.56	193+90.63	1088.71	193+90.76	1088.62	193+90.89	1088.25	193+91.02	1087.89	193+91.15	1087.52	193+91.28	1087.12
	0.40 L	194+03.92	1088.82	194+03.70	1088.60	194+03.83	1088.75	194+03.96	1088.65	194+04.09	1088.29	194+04.22	1087.92	194+04.35	1087.55	194+04.48	1087.15
	0.50 L	194+17.12	1088.83	194+16.90	1088.61	194+17.03	1088.77	194+17.16	1088.65	194+17.29	1088.28	194+17.42	1087.92	194+17.55	1087.55	194+17.68	1087.16
	0.60 L	194+30.32	1088.83	194+30.10	1088.60	194+30.23	1088.76	194+30.36	1088.64	194+30.49	1088.27	194+30.62	1087.90	194+30.75	1087.54	194+30.88	1087.16
	0.70 L	194+43.52	1088.81	194+43.30	1088.59	194+43.43	1088.75	194+43.56	1088.61	194+43.69	1088.25	194+43.82	1087.88	194+43.95	1087.51	194+44.08	1087.14
	0.80 L	194+56.72	1088.79	194+56.50	1088.57	194+56.63	1088.73	194+56.76	1088.59	194+56.89	1088.22	194+57.02	1087.85	194+57.15	1087.49	194+57.28	1087.12
	0.90 L	194+64.92	1088.77	194+69.70	1088.55	194+69.83	1088.71	194+69.96	1088.57	194+70.09	1088.20	194+70.22	1087.83	194+70.35	1087.47	194+70.48	1087.10
Span No.2	0.00 L	194+83.12	1088.77	194+82.90	1088.54	194+83.03	1088.71	194+83.16	1088.56	194+83.29	1088.20	194+83.42	1087.83	194+83.55	1087.46	194+83.68	1087.10
	0.10 L	194+96.32	1088.78	194+96.10	1088.56	194+96.23	1088.72	194+96.36	1088.60	194+96.49	1088.21	194+96.62	1087.84	194+96.75	1087.48	194+96.88	1087.11
	0.20 L	195+09.52	1088.81	195+09.30	1088.58	195+09.43	1088.75	195+09.56	1088.60	195+09.69	1088.24	195+09.82	1087.87	195+09.95	1087.50	195+10.08	1087.13
	0.30 L	195+22.72	1088.83	195+22.50	1088.61	195+22.63	1088.77	195+22.76	1088.64	195+22.89	1088.27	195+23.02	1087.90	195+23.15	1087.54	195+23.28	1087.16
	0.40 L	195+35.92	1088.86	195+35.70	1088.64	195+35.83	1088.80	195+35.96	1088.67	195+36.09	1088.30	195+36.22	1087.93	195+36.35	1087.57	195+36.48	1087.19
	0.50 L	195+49.12	1088.87	195+48.90	1088.65	195+49.03	1088.81	195+49.16	1088.69	195+49.29	1088.32	195+49.42	1087.96	195+49.55	1087.59	195+49.68	1087.20
	0.60 L	195+62.32	1088.87	195+62.10	1088.65	195+62.23	1088.80	195+62.36	1088.70	195+62.49	1088.33	195+62.62	1087.96	195+62.75	1087.59	195+62.88	1087.20
	0.70 L	195+75.52	1088.84	195+75.30	1088.62	195+75.43	1088.77	195+75.56	1088.68	195+75.69	1088.31	195+75.82	1087.94	195+75.95	1087.58	195+76.08	1087.17
	0.80 L	195+88.72	1088.79	195+88.50	1088.57	195+88.63	1088.72	195+88.76	1088.64	195+88.89	1088.28	195+89.02	1087.91	195+89.15	1087.54	195+89.28	1087.12
	0.90 L	196+01.92	1088.73	196+01.70	1088.50	196+01.83	1088.64	196+01.96	1088.59	196+02.09	1088.22	196+02.22	1087.86	196+02.35	1087.49	196+02.48	1087.06
	1.00 L	196+15.12	1088.65	196+14.90	1088.42	196+15.03	1088.56	196+15.16	1088.53	196+15.29	1088.16	196+15.42	1087.79	196+15.55	1087.43	196+15.68	1086.98

SCREED ELEVATIONS *are for the concrete surface before the concrete is placed. Allowance has been made for the anticipated deflection of the structural steel.*

L = Span Length



SCREED LINE LOCATIONS–RIGHT BRIDGE

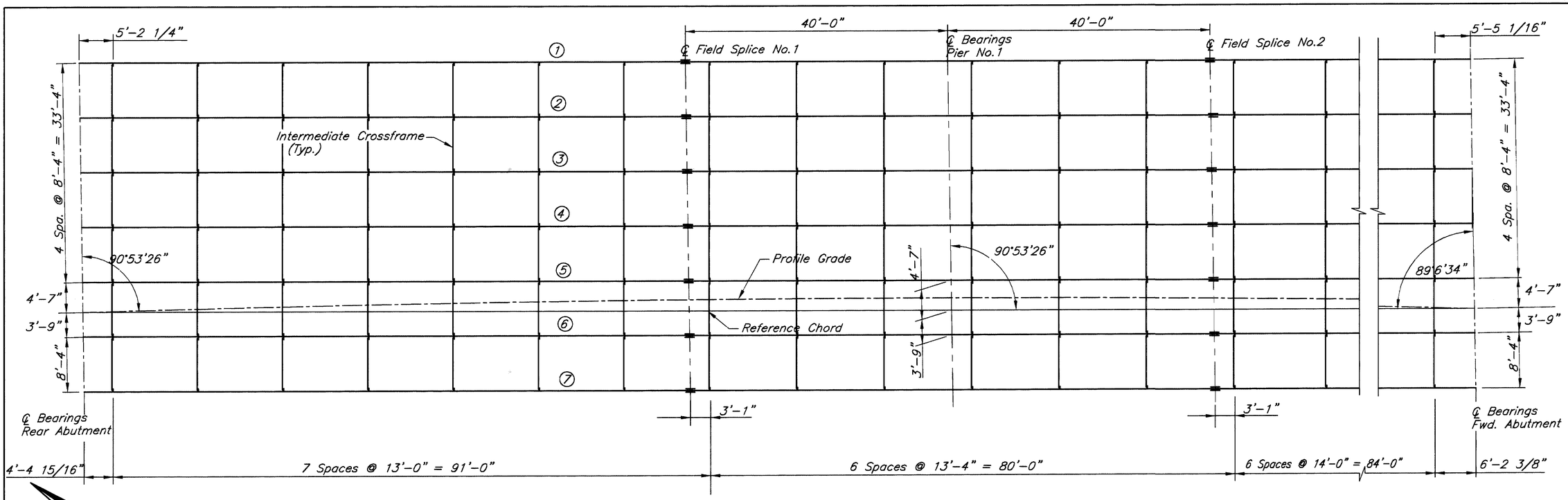
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED

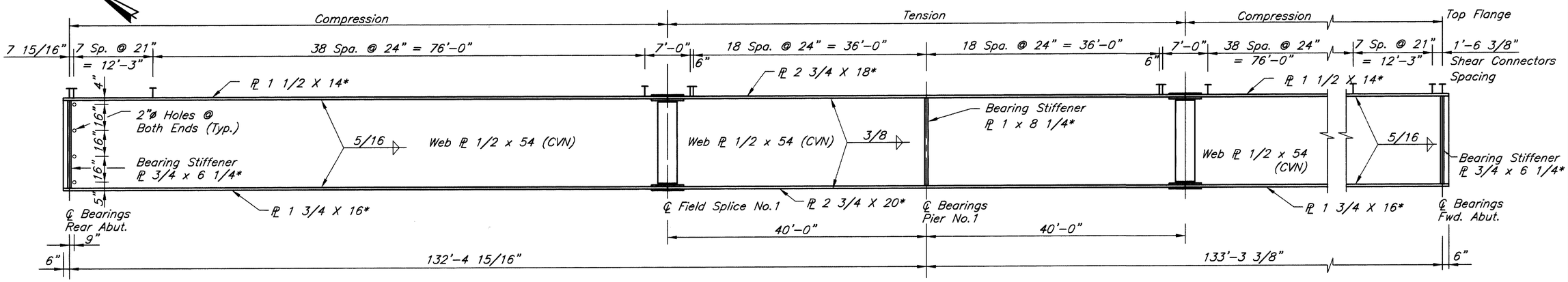
STRUCTURE FILE NO.2400359

SCREED ELEVATIONS
Bridge No. FAY-35-0366 (RIGHT)
U.S.35 OVER I-71

FAY-35-2.57



FRAMING PLAN–LEFT BRIDGE

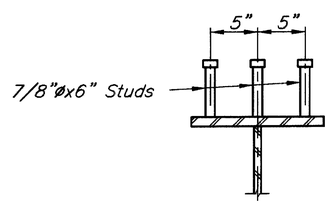


For bottom flange details see section M–M on sheet 16 | 20

GIRDER ELEVATION

DEFLECTION AND CAMBER

	Points	Span No.1 .10	.20	.30	.40	.50	.60	.70	.80	.90
INTERIOR BEAMS	Deflection due to weight of steel	1/4	7/16	9/16	5/8	9/16	7/16	5/16	3/16	1/16
	Deflection due to remaining dead load	5/8	1 1/8	1 1/2	1 5/8	1 1/2	1 1/4	13/16	7/16	1/4
	Adjustment for Vertical curve	3/16	5/16	3/8	7/16	1/2	7/16	3/8	5/16	3/16
	Net Total Camber	1 1/16"	1 7/8"	2 7/16"	2 11/16"	2 9/16"	2 1/8"	1 1/2"	15/16"	1/2"



SHEAR CONNECTOR DETAIL

NOTES :

WELDED ATTACHMENT of supports for concrete deck finishing machine may be made to areas of the flanges designated "Compression". Attachments shall not be made to areas designated "Tension". Fillet welds to compression flanges shall be not closer than 1" from edge of the flange, be not more than 2" long and be not smaller than the minimum size required by AASHTO.

* **CHARPY V–NOTCH TOUGHNESS REQUIREMENT**

All girder components, i.e., flange plates, web plates and splice plates shall meet minimum notch toughness requirements as specified in 711.01 of CMS.

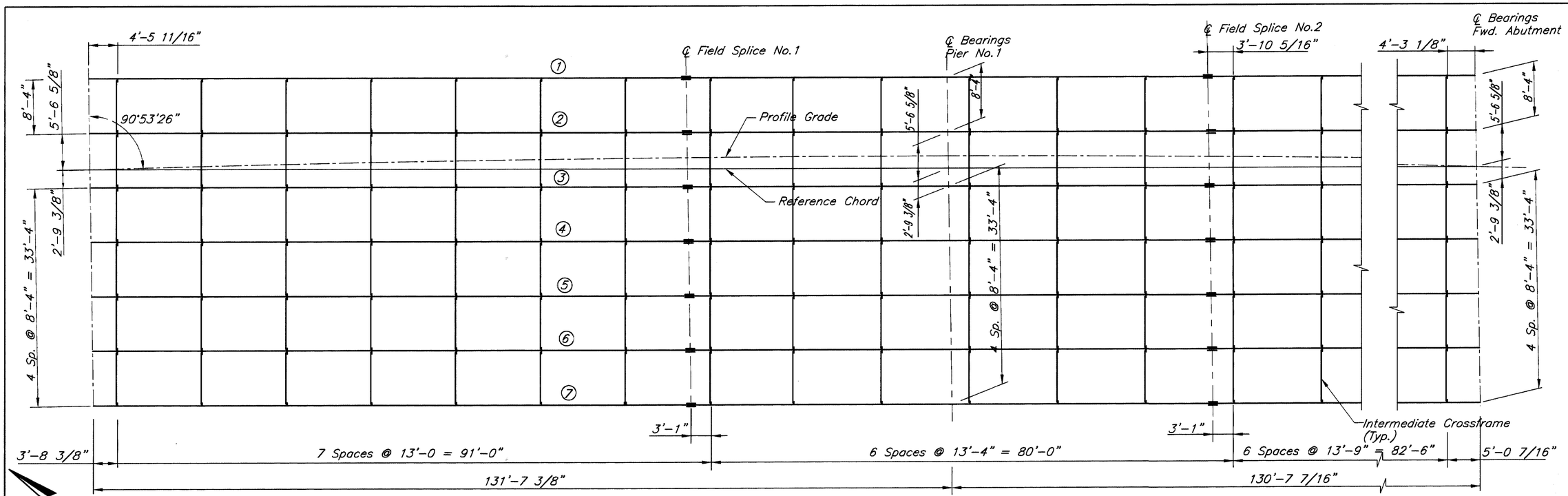
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

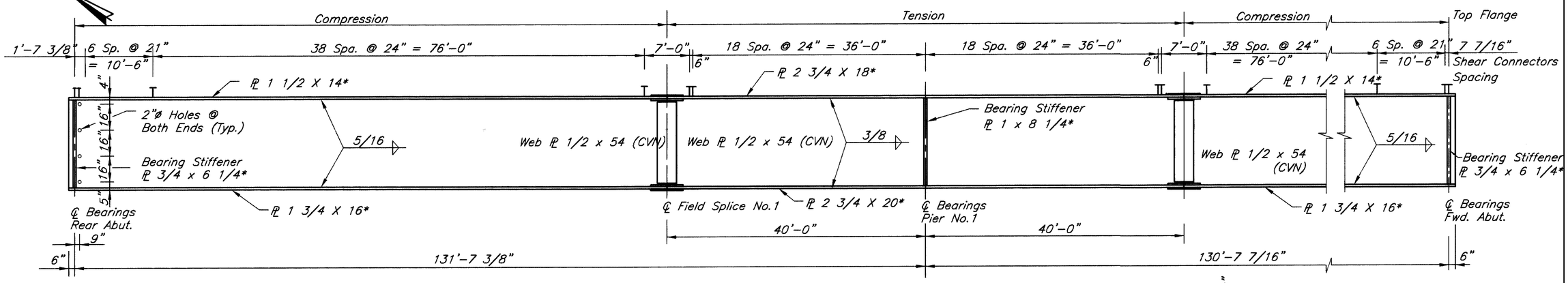
STRUCTURE FILE 2400359

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0366 (LEFT)
U.S.R.35 OVER I.R.71

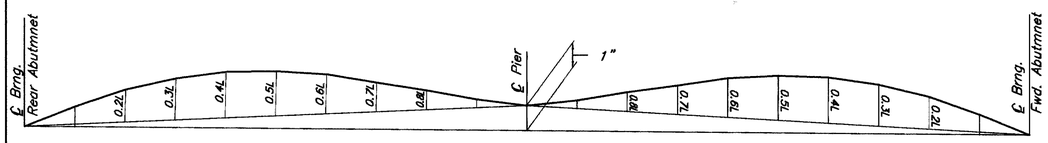
FAY-35-2.57



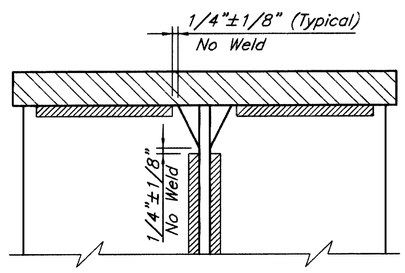
FRAMING PLAN – RIGHT BRIDGE



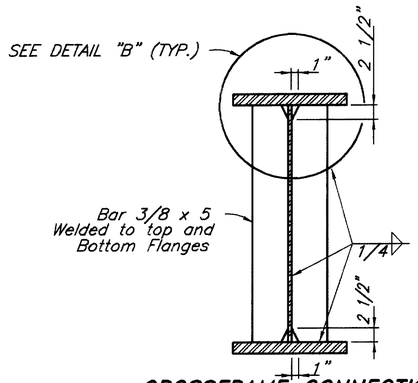
GIRDER ELEVATION



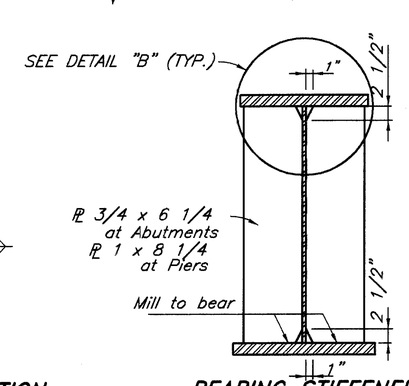
CAMBER DIAGRAM



DETAIL "B"



CROSSFRAME CONNECTION PLATE DETAIL



BEARING STIFFENER DETAIL

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE 2400359

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0366 (RIGHT)
U.S.R.35 OVER I.R.71

FAY-35-2.57

17 / 20

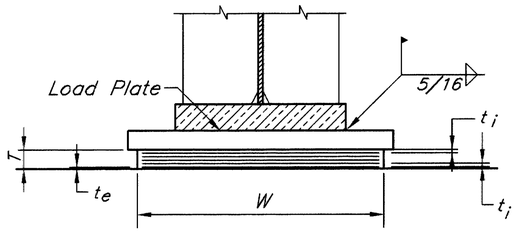
433
522

LAMINATED ELASTOMERIC BEARINGS

LOCATION	DIMENSIONS (Inch)					N	STEEL LOAD PLATE	REACTIONS		MAXIMUM DESIGN LOAD
	L	W	t_i	t_e	T			DL	LL	
Pier Expansion Brngs.	20	26	0.36	0.26	2	4	21" x 27" x 2"	330 k	102 k	432 k

t_i = Thickness of Internal Layer
t_e = Thickness of External Layer
T = Total Thickness of Elastomeric Bearing

N = No. of Steel Laminates
Internal Steel Laminate Thickness = 0.0747"
Durometer of Elastomer = 50 Durometer



LAMINATED ELASTOMERIC EXPANSION BEARING

LOAD PLATE

The steel load plate shall be the same material as the attached structural steel and be similarly cleaned and coated. Surface preparation and priming shall be done in the shop and be included in the price bid for the bearings. Field coats shall be included in the price bid for painting main structural steel.

The steel load plate shall be bonded by vulcanization to the elastomer during the molding process.

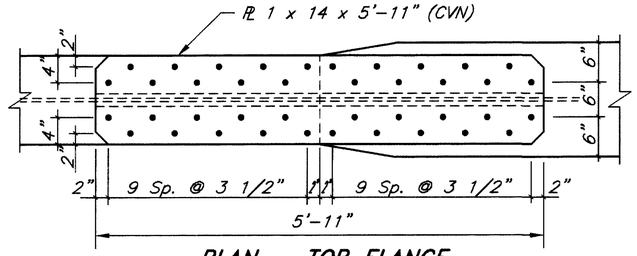
Welding of the load plate to the superstructure shall be controlled so that the plate temperature at the elastomer bonded surface shall not exceed 300° F as determined by the use of pyrometric sticks or other temperature monitoring devices.

BASIS OF PAYMENT

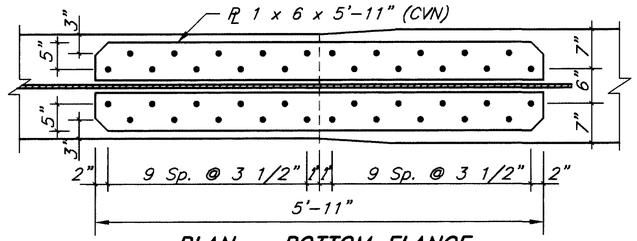
The unit bid price shall include all materials, labor and incidentals necessary to furnish and install laminated elastomeric bearings either fixed or expansion. Payment will be made at the contract price for Item 516, each, Laminated Elastomeric Bearings.

ELASTOMERIC BEARINGS

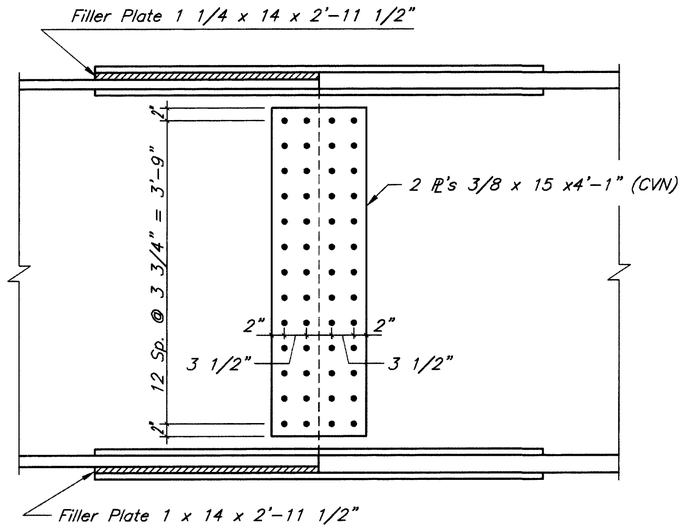
Elastomeric bearings shall comply with Item 516 and articles 18.2.5 through 18.2.8, Section 18, Bearing Devices, Division II, Construction, of the AASHTO Specifications for Highway Bridges. Bearings shall be Grade 3, 50-durometer elastomer and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the price bid for the bearings.



PLAN - TOP FLANGE



PLAN - BOTTOM FLANGE



TYPICAL FIELD SPLICE DETAILS

HIGH STRENGTH BOLTS

High strength bolts shall be 1" diameter A325 unless otherwise noted.

*** CHARPY V-NOTCH TOUGHNESS REQUIREMENT**

Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in 711.01 of CMS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.2400359

BEARING & SPLICE DETAILS
Bridge No. FAY-35-0366 L.& R.
U.S.35 Over I-71

FAY-35-2.57

REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
			REAR ABUTMENT LEFT BRIDGE						
A401	28	9–0	168	1	1–9	2–6			
A501	57	11–1	659	1	2–8	2–7			
A502	57	12–0	713	2	2–8	4–9 1/2			
A503	20	10–0	209	2	2–2	4–0 1/2			
A504	16	30–0	501	STR					
A505	8	26–2	218	STR					
A510	12	13–1	164	STR					
A511	10	14–11	156	2	2–2	6–6			
A514	8	11–7	97	STR					
A515	6	3–3 to	38	STR					2 Sets of 3
		8–9							Vary by 2'–9"
A516	2	12–6	26	STR					
A517	10	8–11 to	140	2	2–2	3–6 to			1 Set of 10
		17–11				8–0			Vary "B" by 6"
A601	54	10–9	872	4					
A801	24	30–0	1922	STR					
			5883	**LBS**					
			FORWARD ABUTMENT LEFT BRIDGE						
A401	28	9–0	168	1	1–9	2–6			
A500	9	11–1 to	117	1	2–8 to	2–7			1 Sets of 9
		13–9			4–0				Vary "A" by 2"
A501	48	11–1	555	1	2–8	2–7			
A502	57	12–0	713	2	2–8	4–9 1/2			
A503	20	10–0	209	2	2–2	4–0 1/2			
A504	16	30–0	501	STR					
A505	8	26–2	218	STR					Bend in Field as Req.
A520	8	11–7	97	STR					
A521	6	3–9 to	38	STR					2 Sets of 3
		9–0							Vary by 3'–0"
A522	2	12–4	26	STR					
A523	10	8–5 to	135	2	2–2	3–3 to			1 Set of 10
		17–5				7–9			Vary "B" by 6"
A525	10	14–11	156	2	2–2	6–6			
A526	12	13–1	164	STR					
A601	54	10–9	872	4					
A821	18	30–0	1442	STR					
A822	6	30–0	481	0	16–10	13–2	1–4		
			5892	**LBS**					

Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
			REAR ABUTMENT RIGHT BRIDGE						
A401	28	9–0	168	1	1–9	2–6			
A501	57	11–1	659	1	2–8	2–7			
A502	57	12–0	713	2	2–8	4–9 1/2			
A503	20	10–0	209	2	2–2	4–0 1/2			
A504	16	30–0	501	STR					
A505	8	26–2	218	STR					
A545	4	11–7	48	STR					
A546	6	3–3 to	39	STR					2 Sets of 3
		9–3							Vary by 3'–0"
A547	2	12–4	26	STR					
A548	10	5–11 to	109	2	2–2	2–0 to			1 Set of 10
		14–11				6–6			Vary "B" by 6"
A549	12	13–1	164	STR					
A550	10	17–3	180	2	2–2	7–8			
A601	54	10–9	872	4					
A841	24	30–0	1922	STR					
			5828	**LBS**					
			FORWARD ABUTMENT RIGHT BRIDGE						
A401	28	9–0	168	1	1–9	2–6			
A500	9	11–1 to	117	1	2–8 to	2–7			1 Set pf 9
		13–9			4–0				Vary "A" by 2"
A501	48	11–1	555	1	2–8	2–7			
A502	57	12–0	713	2	2–8	4–9 1/2			
A503	20	10–0	209	2	2–2	4–0 1/2			
A530	8	26–2	218	STR					Bend in Field as Req.
A531	16	30–0	501	STR					
A532	12	13–1	164	STR					
A533	10	17–5	182	2	2–2	7–9			
A534	4	11–7	48	STR					
A535	6	3–0 to	44	STR					2 Sets of 3
		11–0							Vary by 4'–0"
A536	2	12–4	26	STR					
A537	10	5–11 to	109	2	2–2	2–0 to			1 Set of 10
		14–11				6–6			Vary "B" by 6"
A601	54	10–9	872	4					
A831	18	30–0	1442	STR					
A832	6	30–0	481	0	16–10	13–2	1–4		
			5849	LBS					

ALL BARS SHALL BE EPOXY COATED

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 4-22-99

CHECKED VK | REVISED

STRUCTURE FILE NO.2400359

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0366
US 35 OVER I-71

FAY-35-2.57

19 / 20

435
522

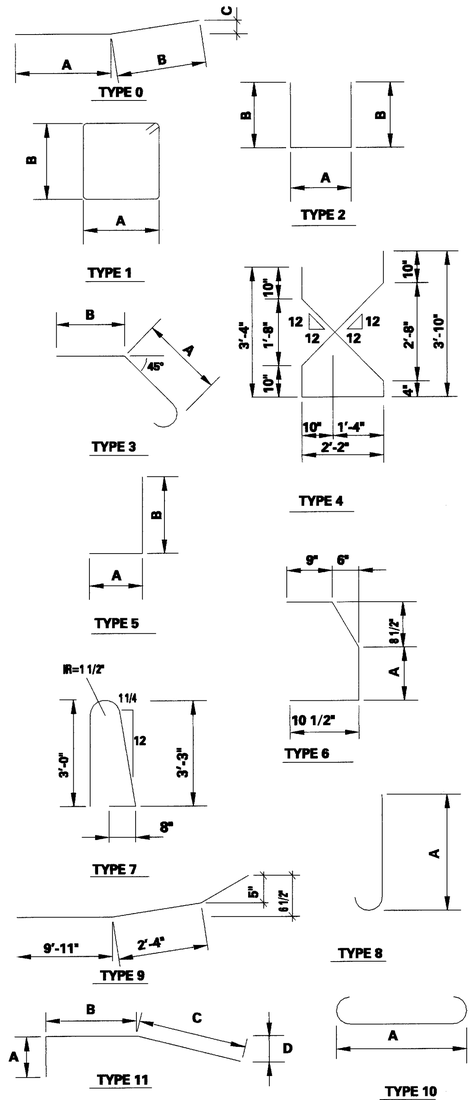
REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				PIERS (BOTH BRIDGES)					
P501	16	6-6	108	STR					
P502	8	27-4	228	STR					
P601	232	12-0	4182	1	2-0 1/2	3-8			
P801	80	11-4	2421	10	9-6				
P1001	80	14-4	4934	10	11-6				
P1002	80	9-3	3184	5	1-10	7-9			
P1003	10	19-6	839	STR					
P1004	10	20-3	871	STR					
P1005	10	21-0	904	STR					
P1006	10	21-0	904	STR					
P1007	12	30-0	1549	STR					
P1008	20	20-0	1721	STR					
P1011	10	18-5	792	STR					
P1012	10	19-0	818	STR					
P1013	10	19-8	846	STR					
P1014	10	19-8	846	STR					
P1101	12	33-8	2146	5	30-6	3-6	30-6	3-6	
P1102	12	28-0	1785	5	25-3	3-1	25-3	3-1	
P1103	12	33-8	2147	11	3-6	19-0	11-6	0-6	
P1104	12	28-0	1785	11	3-1	18-6	6-9	0-4	
Spirals			2673						
			35683	LBS					

Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
			SUPERSTRUCTURE (AT ABUTMENT, BOTH BRIDGES)						
S510	44	29-4	1346	STR					
S610	40	14-2	851	2	2-2	6-2			
S611	216	7-6	2433	2	2-2	2-10			
S612	216	7-6	2433	2	1-8	3-1			
S613	32	29-6	1418	STR					
S614	16								
S620	32	14-2	681	2	2-2	6-2			
S621	32	13-4	641	2	2-2	5-9			
S622	32	12-8	609	2	2-2	5-5			
S623	32	11-10	569	2	2-2	5-0			
S624	48	10-8	769	2	2-2	4-5			
S801	32	30-0	2563	STR					
S802	44	26-0	3054	STR					
S803	24	12-0	769	STR					
S804	156	4-8	1944	3	2-4	1-5			
			20080	LBS					
			SUPERSTRUCTURE (DECK, BOTH BRIDGES)						
S401	1422	30-0	28497	STR					
S402	79	12-4	651	STR					
S403	79	9-0	475	STR					
S501	1098	30-0	34356	STR					
S502	69	15-4	1103	STR					
S503	69	12-0	864	STR					
S504	1864	28-4	55084	STR					
S505	856	2-6	2232	5	10 1/2	1-9			
S506	856	3-5	3050	6	1-0				
S507	856	7-1	6324	7					
S508	142	30-0	4443	STR					
S509	142	22-8	3357	STR					
S601	932	27-0	37795	STR					
S602	932	30-0	41986	STR					
			220228	LBS					

ALL BARS SHALL BE EPOXY COATED



MARK	NO.	LENGTH	WEIGHT	SHAPE	REMARKS
SP401	1	17-6	329	BT.	
SP402	1	18-3	342	BT.	
SP403	1	18-11	354	BT.	
SP404	1	18-11	354	BT.	
SP405	1	16-4	308	BT.	
SP406	1	17-0	320	BT.	
SP407	1	17-9	333	BT.	
SP408	1	17-9	333	BT.	
			2673	LBS.	

SP401 - Core Diameter 30" c/c

Pitch 4 1/2"

Other Details in Accordance with C.R.S.I. Standard Practice.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Four steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 3.20 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

CHECKED VK | REVISED

STRUCTURE FILE NO.2400359

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0366
US 35 OVER I-71

FAY-35-2.57

Point of Min. Vertical Clearance
Sta.76+34.30 = Sta.229+80.52

Sta.75+55.23

Begin Approach Slab
Sta.78+61.13

Sta.78+53.03

B-22 B-21 B-19 B-20 B-18

41°-08'-24"

1'-6" 36'-0" 24'-0" 1'-6"

24'-0" 12'-0"

℄ Ex. U.S.R.35

S 59°-22'-43" E

℄ Survey & Construction

End Approach Slab
Sta.76+09.13

Sta.76+06.75

Req. Min.30'-0"
Actual = 39'-1"

Req. Min.30'-0"
Actual = 36'-2"

Sta.79+04.56

30'-0" 24'-0" 12

Sta. 231+71.97 Prop. U.S.R. 35
= Sta.77+35.13 Ex. U.S.R.35

B.M.-TR 2
Iron Pin With Cap
Sta.67+57.90, 31.4' Right
Elev. = 1061.230

EARTHWORK LIMITS shown are approximate. Actual slopes shall conform to plan cross-sections.

Indicates Boring Location

PLAN

DESIGN TRAFFIC

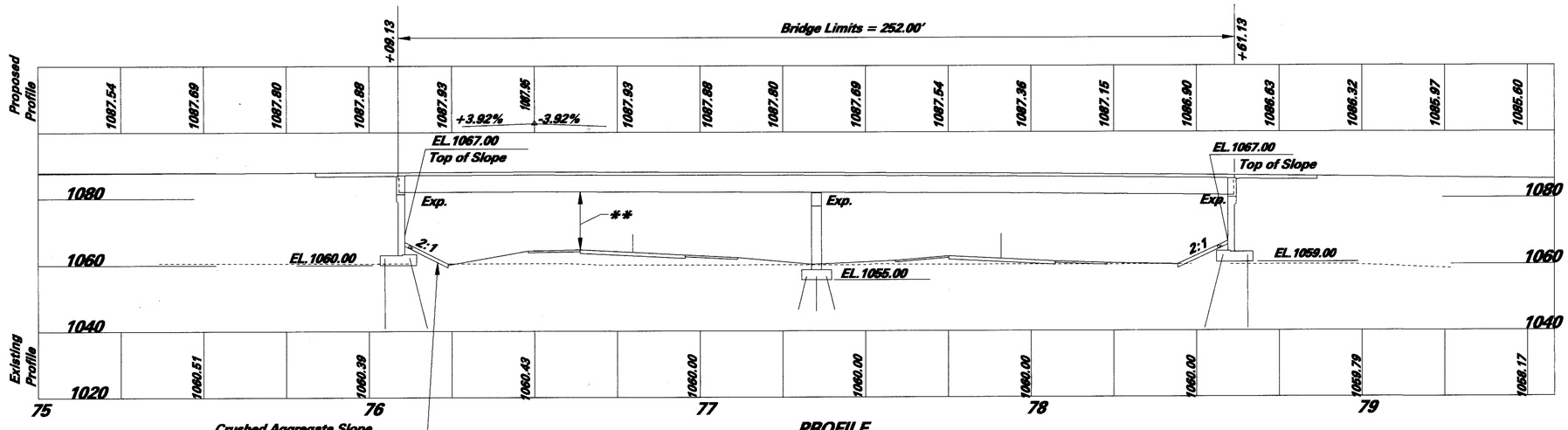
1997 ADT 5060
2017 ADT 6390
26 % Trucks

PILE DATA

Location	Type	Estimated Pile Length
Abutments	14"0 Conc.	51'
Piers	12"0 Conc.	52'

VERTICAL CURVE DATA

P.V.I. Sta. 76+50.00
P.V.I. EL. 1102.65
G_1 = +3.92%
G_2 = -3.92%
L = 1500'



Crushed Aggregate Slope Protection, 1'-0" Thick (Typ.) Included with Roadway Quantities for Payment.

PROFILE

On ℄ Survey & Construction

** Required Min. Vertical Clearance = 16.5'
Preferred Min. Vertical Clearance = 17.0'
Provided Min. Vertical Clearance = 17.08'

PROPOSED STRUCTURE

TYPE : Continuous Steel Girders (A572-50) on Reinf. Conc. Substructure
SPAN: 2 Spans @ 125'-0" c/c Bearings
ROADWAY: 60'-0" Toe/Toe Parapets
LOADING: HS20-44 (Case II) and the Alternate Military Loading
SKEW: 41°-08'-24" R.F.
WEARING SURFACE: Monolithic Concrete
APPROACH SLABS: 25' Long (AS-1-81)
ALIGNMENT: Tangent
CROWN : 0.0156:1
LATITUDE : 39°-36'-51.25"N
LONGITUDE : 83°-35'-48.71"W
USGS QUADRANGLE : MILLEDGEVILLE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

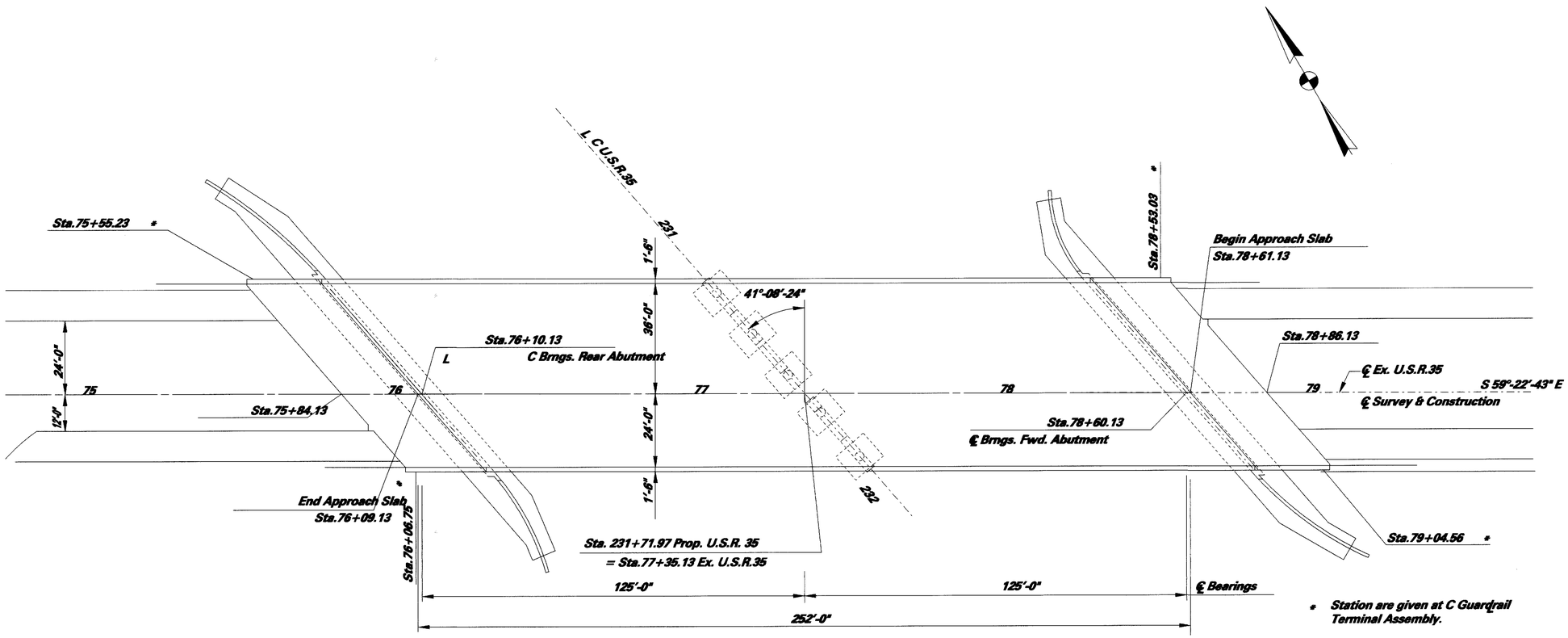
DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP | REVISED
STRUCTURE FILE NO. 3400686

FAYETTE COUNTY
STA.76+09.13
STA.78+61.13

SITE PLAN
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED U.S.R.35

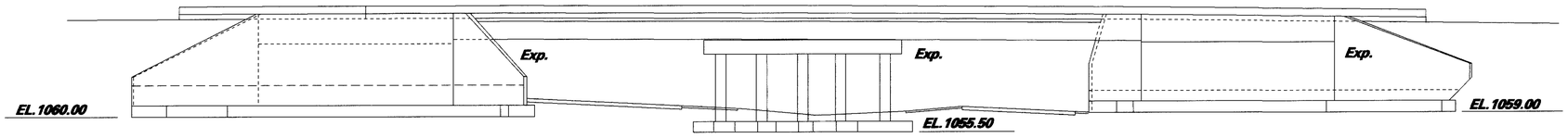
FAY-35-2.56

437
522



* Station are given at C Guardrail Terminal Assembly.

GENERAL PLAN



ELEVATION

Piles not shown

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURAL FILE NO. 2400098	
SP			

GENERAL PLAN & ELEVATION
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED U.S.R.35

FAY-35-2.56

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844	-	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C	-	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL	-	ASTM A615, A616 OR A617 GRADE 60 UNIT STRESS 24,000 PSI
STRUCTURAL STEEL	-	ASTM A572-YIELD STRENGTH 50,000 P.S.I.

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81	DATED 9-15-94	SHEET NO. 1. AND 2.
BR-1	DATED 1-6-99	
GSD-1-96	DATED 2-12-97	

AND TO SUPPLEMENTAL SPECIFICATION :

816	DATED 4-21-97
842	DATED 1-6-99
844	DATED 1-6-99
863	DATED 10-12-99

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, 2 1/2" CONCRETE COVER AT TOP AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 120 TONS PER PILE FOR THE 14" CIP ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 88 TONS PER PILE FOR THE 12" CIP PIER PILES.

ABUTMENT PILES:
122 PILES 51 FEET LONG, ESTIMATED LENGTH
122 PILES OF ORDER LENGTH 56 FEET LONG
61 SPLICES

PIER PILES:
45 PILES 52 FEET LONG, ESTIMATED LENGTH
45 PILES OF ORDER LENGTH 57 LONG
22 SPLICES

ABUTMENT BACKFILL

ABUTMENT BACKFILL ABOVE BRIDGE SEAT SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

CONSTRUCTION CONSTRAINTS:

ALL EMBANKMENT MATERIAL FOR FILLING THE VOID CREATED BY EXCAVATION FOR THE ABUTMENT FOOTINGS SHALL BE 203 GRANULAR EMBANKMENT MATERIAL. AFTER THE FOOTING AND THE BREASTWALL HAVE BEEN CONSTRUCTED, THE VOID BEHIND EACH ABUTMENT SHALL BE FILLED UP TO THE BEAM SEAT ELEVATION AND FROM THE BEAM SEAT UP ON A 1:1 SLOPE TO THE SUBGRADE ELEVATION PRIOR TO CONSTRUCTING THE BACKWALL AND SETTING THE BEAMS ON THE ABUTMENT.

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.08 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

CONCRETE PARAPETS:

WITHIN 48 HOURS AFTER PLACEMENT OF PARAPET CONCRETE SAWCUT 1 INCH DEEP JOINTS INTO THE CONCRETE PARAPET AT LOCATIONS AS DETAILED IN THE PLANS. THE SAW CUT SHALL BE MADE IN THE COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK, AND THE COMPLETED SAWCUT SHALL BE FILLED WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION TT-S-00227E. THE BOTTOM HALF INCH OF THE ONE INCH DEEP SAWED JOINT IN BOTH THE INSIDE AND OUTSIDE FACES OF THE PARAPET SHOULD BE LEFT UNSEALED TO ALLOW ANY WATER WHICH MAY ENTER THE JOINT TO ESCAPE. ALL LABOR, TOOLS, EQUIPMENT, MATERIAL AND INCIDENTALS NECESSARY TO COMPLETE THIS WORK ARE INCLUDED UNDER ITEM 842 CLASS S CONCRETE, SUPERSTRUCTURE.

ITEM 203 EMBANKMENT AS PER PLAN

ALL FILL MATERIAL FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT PLACED BETWEEN STATIONS 74+75 TO 79+80 AND FOR FILLING THE EXCAVATION VOID CREATED BY ITEM 503, SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

ITEM 842 CLASS C CONCRETE, ABUTMENT
NOT INCLUDING FOOTING, AS PER PLAN

INSTALL A 3 FOOT WIDE STRIP, 3/32 INCH THICK, GENERAL PURPOSE HEAVY DUTY NEOPRENE SHEET WITH NYLON FABRIC REINFORCEMENT AT LOCATIONS SHOWN IN THE PLANS. SECURE THE 3 FOOT WIDE NEOPRENE SHEETING TO THE CONCRETE WITH 1 1/4" X 3/32" X 1/4" (LENGTH X SHANK DIAMETER X HEAD DIAMETER) #10 GALVANIZED SCREWS THROUGH A 1 INCH OUTSIDE DIAMETER, #10 GAGE GALVANIZED WASHER. MAXIMUM FASTENER SPACING IS 9 INCHES. OTHER SIMILAR GALVANIZED DEVICES WHICH WILL NOT DAMAGE EITHER THE NEOPRENE OR THE CONCRETE MAY BE USED SUBJECT TO THE APPROVAL OF THE ENGINEER.

CENTER THE NEOPRENE STRIPS ON ALL JOINTS. FOR HORIZONTAL JOINTS, SECURE THE HORIZONTAL NEOPRENE STRIP BY USING A SINGLE LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE TOP OF THE NEOPRENE STRIP. FOR THE VERTICAL JOINTS SECURE THE VERTICAL NEOPRENE STRIP BY USING A SINGLE VERTICAL LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE VERTICAL EDGE OF THE NEOPRENE STRIP NEAREST TO THE CENTERLINE OF ROADWAY. FOR VERTICAL JOINTS, INSTALL 2 ADDITIONAL FASTENERS AT 6 INCHES CENTER TO CENTER ACROSS THE TOP HALF OF THE NEOPRENE STRIP ON THE SAME SIDE OF THE VERTICAL JOINT AS WHERE THE SINGLE VERTICAL ROW OF FASTENERS ARE LOCATED.

THE VERTICAL NEOPRENE STRIPS SHOULD COMPLETELY OVERLAP THE HORIZONTAL STRIPS DUE TO MATERIAL MANUFACTURING SHALL BE AT LEAST ONE FOOT IN LENGTH, IF NOT VULCANIZED OR ADHESIVED, OR 6 INCHES IN LENGTH IF THE LAP IS VULCANIZED OR ADHESIVED. NO LAPS ARE ACCEPTABLE IN VERTICALLY INSTALLED NEOPRENE STRIPS. AS PER PLAN.

PAYMENT FOR LABOR, MATERIALS AND INSTALLATION OF THESE ITEMS SHALL BE INCLUDED IN ITEM 842 CLASS C CONCRETE, ABUTMENT, NOT INCLUDING FOOTING, AS PER PLAN.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 5/13, 6/13 AND 8/13.
SEE PROPOSAL NOTE FOR SURFACE PREPARATION REQUIREMENTS, APPLICATION RATES, MATERIAL REQUIREMENTS AND APPLICATION PROCEDURES.

ITEM 507 14" PILES, AS PER PLAN

PILE WALL THICKNESS: THE RESPONSIBILITY OF CHOOSING AND PROVIDING A SATISFACTORY PILE WALL THICKNESS FOR THIS PROJECT SHALL BE BORNE BY THE CONTRACTOR EXCEPT THAT THE PILE WALL THICKNESS SHALL NOT BE LESS THAN 0.22 INCHES. IF A PILE WALL THICKNESS GREATER THAN 0.22 INCHES IS NECESSARY TO RESIST THE PILE INSTALLATION DRIVING STRESS, THE CONTRACTOR SHALL MAKE THIS DETERMINATION AND SHALL FURNISH A PILE WITH AN ACCEPTABLE WALL THICKNESS. IF MONOTUBE PILES ARE USED, THE MINIMUM WALL THICKNESS SHALL BE 0.17 INCHES.

ITEM 507 14" PILES, AS PER PLAN

PILE HAMMER: THE PILE HAMMER USED TO INSTALL THE 14" PILES SHALL HAVE A STATE'S ENERGY RATING OF NOT LESS THAN 18500 FOOT-POUNDS. THIS REQUIREMENT DOES NOT RELIEVE THE CONTRACTOR FROM 108.05 WHICH STATES THAT THE CONTRACTOR IS TO PROVIDE SUFFICIENT EQUIPMENT FOR PROSECUTING THE REQUIRED WORK. REFER TO "ODOT'S MANUAL OF PROCEDURES FOR STRUCTURES" TO OBTAIN THE STATE'S ENERGY RATING.

ITEM 503, UNCLASSIFIED EXCAVATION ***, AS PER PLAN

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN 6 INCH LIFTS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1632 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
VK			

STRUCTURE FILE NO.2400306

GENERAL NOTES
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57

May 24, 1999 Q:\BRIDGE\Fay35\3566dgn.dwg

ESTIMATED QUANTITIES

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	ABUT.	PIER	SUPER.	GEN.
503	21101	1473	CU YD	UNCLASSIFIED EXCAVATION, AS PER PLAN	1317	156		
505	11100	LUMP	LUMP	PILE DRIVING EQUIPMENT MOBILIZATION				LUMP
507	00501	2340	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN, AS PER PLAN		2340		
507	00551	2565	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED, AS PER PLAN		2565		
507	00601	6222	LIN.FT.	14" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN, AS PER PLAN	6222			
507	00651	6832	LIN.FT.	14" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED, AS PER PLAN	6832			
507	50500	83	EACH	STEEL PILE SPLICES	61	22		
SPECIAL	51267500	698	SQ YD	SEALING OF CONCRETE SURFACES *		197	501	
SPECIAL	51267502	488	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY)	488			
516	13900	178	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	178			
516	44001	14	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE. 12" x 18" x 1 5/8" NEOPRENE WITH 1 1/2" x 13" x 19" STEEL LOAD PLATE, AS PER PLAN *	14			
516	44101	7	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE. 18" x 26" x 2 1/4" NEOPRENE WITH 2" x 19" x 33" STEEL LOAD PLATE, AS PER PLAN *		7		
518	21200	358	CU YD	POROUS BACKFILL WITH FILTER FABRIC	358			
523	11100	6	HOURS	DYNAMIC LOAD TEST				6
816	00600	LUMP	SUM	FIELD PAINTING OF NEW STEEL, INTERMEDIATE AND FINISH COAT SYSTEM IZEU			LUMP	
844	48000	574	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (DECK)			574	
844	48020	78	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (PARAPET)			78	
844	49000		LUMP	HIGH PERFORMANCE CONCRETE, TRIAL MIX				
844	49010		LUMP	HIGH PERFORMANCE CONCRETE, TESTING				
842	41000	61	CU YD	CLASS C CONCRETE, PIER ABOVE FOOTING		61		
842	44101	385	CU YD	CLASS C CONCRETE, ABUTMENT NOT INCLUDING FOOTING, AS PER PLAN	385			
842	46500	549	CU YD	CLASS C CONCRETE, FOOTING	471	78		
863	10080	LUMP	SUM	STRUCTURAL STEEL MEMBERS LEVEL FOUR (4) FABRICATION			LUMP	
863	20000	2793	EACH	WELDED STUD SHEAR CONNECTOR			2793	

Computed by: S.P. Date: 8-8-94

Checked by: V.K. Date: 9-8-94

* SEE PROPOSAL NOTE

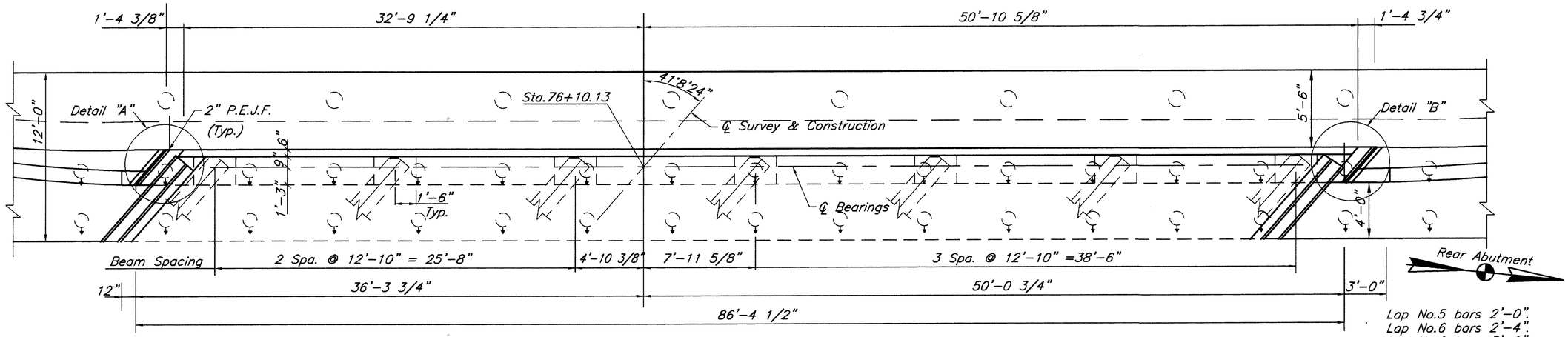
May 24, 1999 Q:\BRIDGE\Fay35\3566dqn.dwg

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO.2400308	
VK			

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED U.S.35

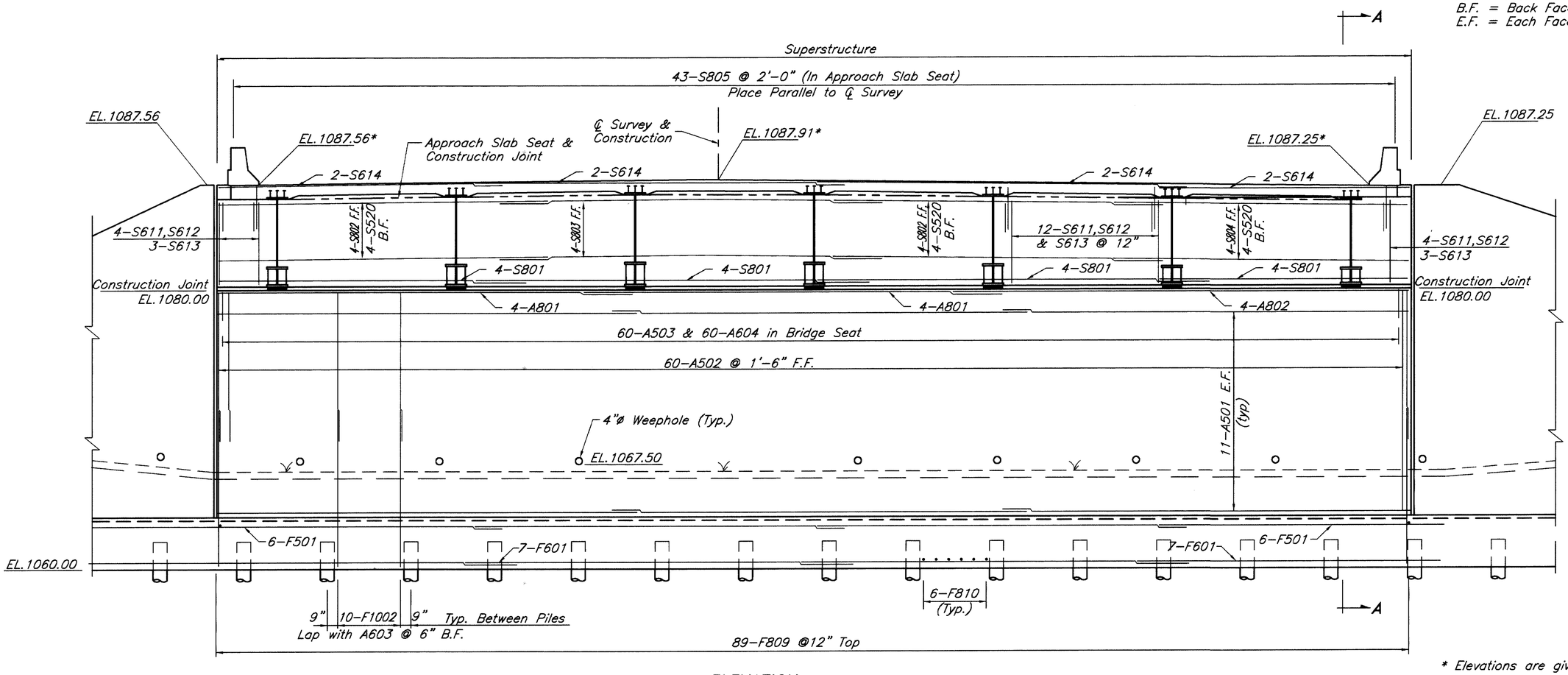
FAY-35-2.57



Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

PLAN–REAR ABUTMENT

ABBREVIATIONS
F.F. = Front Face
B.F. = Back Face
E.F. = Each Face



ELEVATION

* Elevations are given @ ℄ Bearings

For Section A–A see sheet 9 | 18
For Pile Layout see sheet 10 | 18
For Details "A"& "B" see sheet 9 | 18

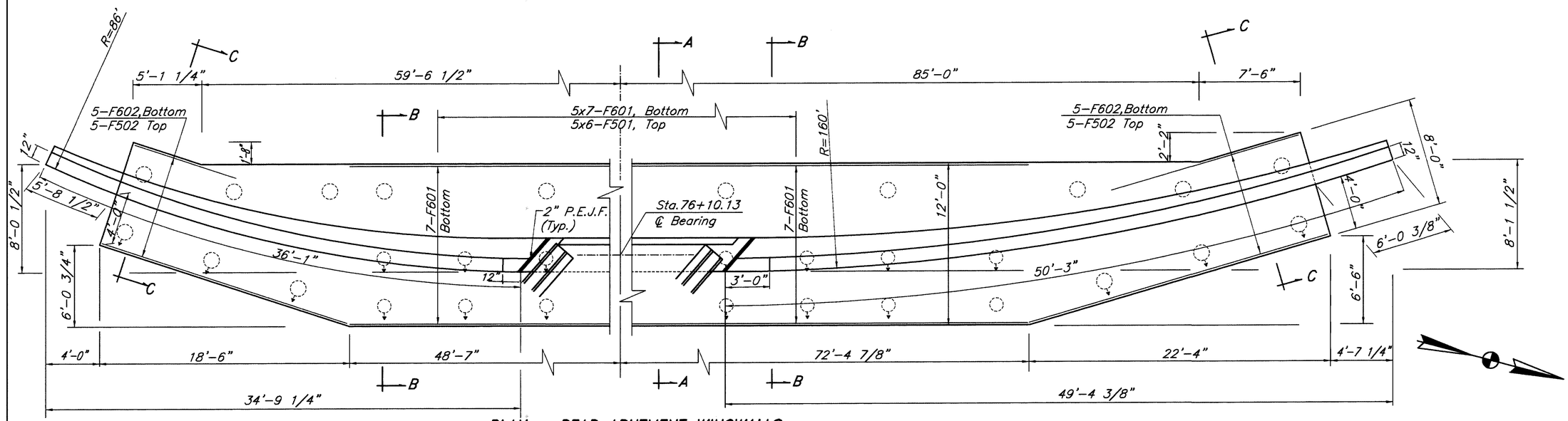
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

STRUCTURE FILE NO.2400308

REAR ABUTMENT DETAILS
Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

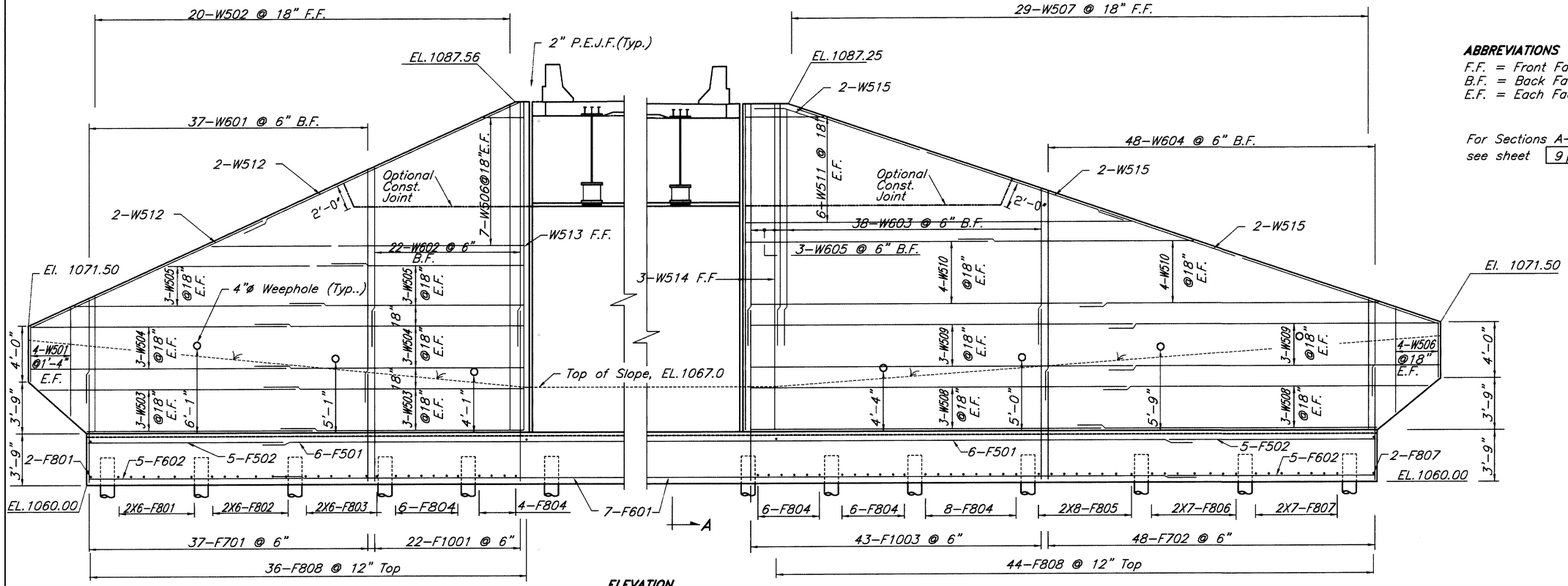
FAY-35-2.57



PLAN – REAR ABUTMENT WINGWALLS

For Pile Layout See Sheet 10/18

Lap No.5 bars 2'-0"
Lap No.6 bars 2'-4"
Lap No.8 bars 3'-9"



ELEVATION

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face

For Sections A–A & B–B see sheet 9/18

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

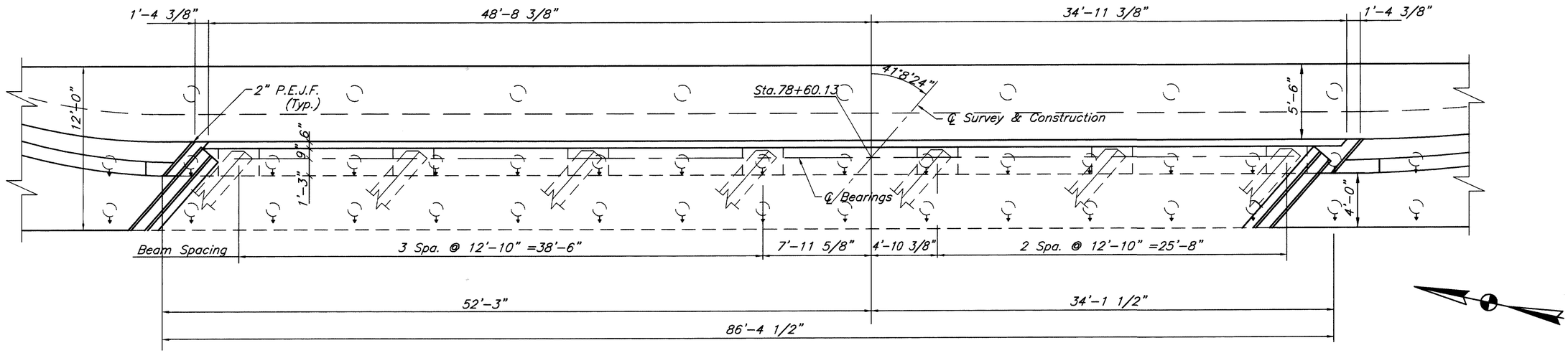
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.2400308

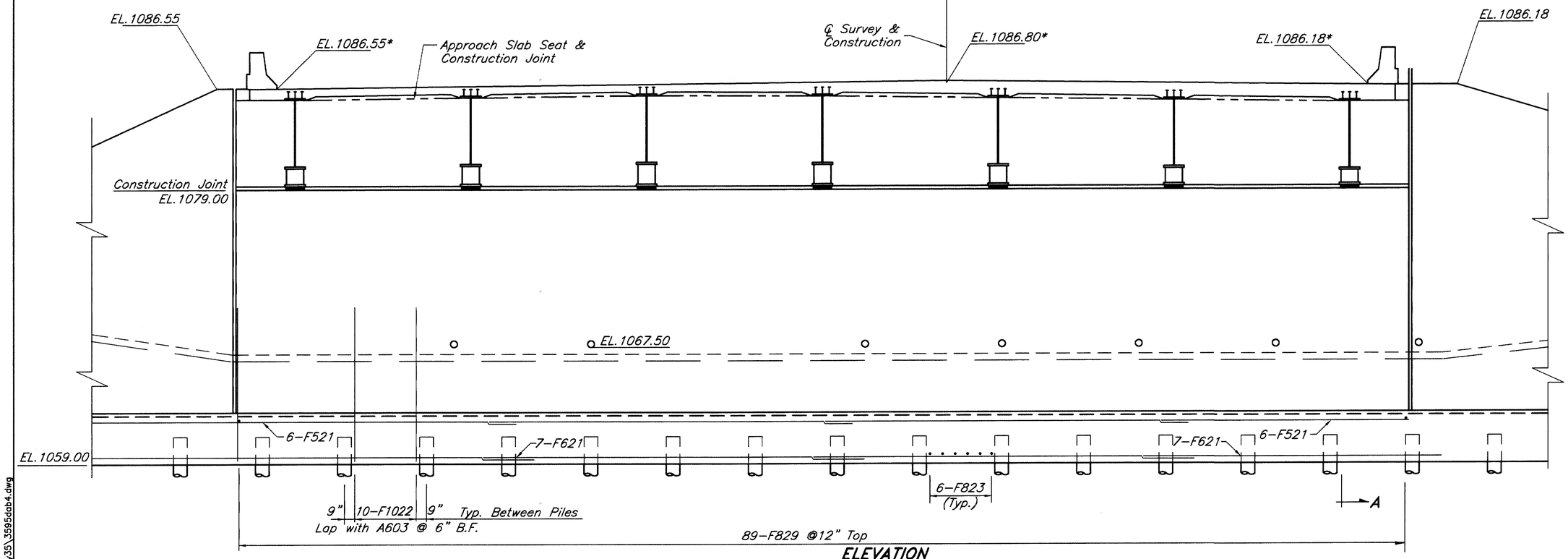
REAR ABUTMENT WINGWALLS

Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57



PLAN-FORWARD ABUTMENT



ELEVATION

* Elevations are given @ ℄ Bearings

For notes, minimum lap lengths and Sections A-A see sheet 9 | 18

For Pile Layout See Sheet 10 | 18

Abutment stem & diaphragm reinforcing details are identical to those at the Rear Abutment.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

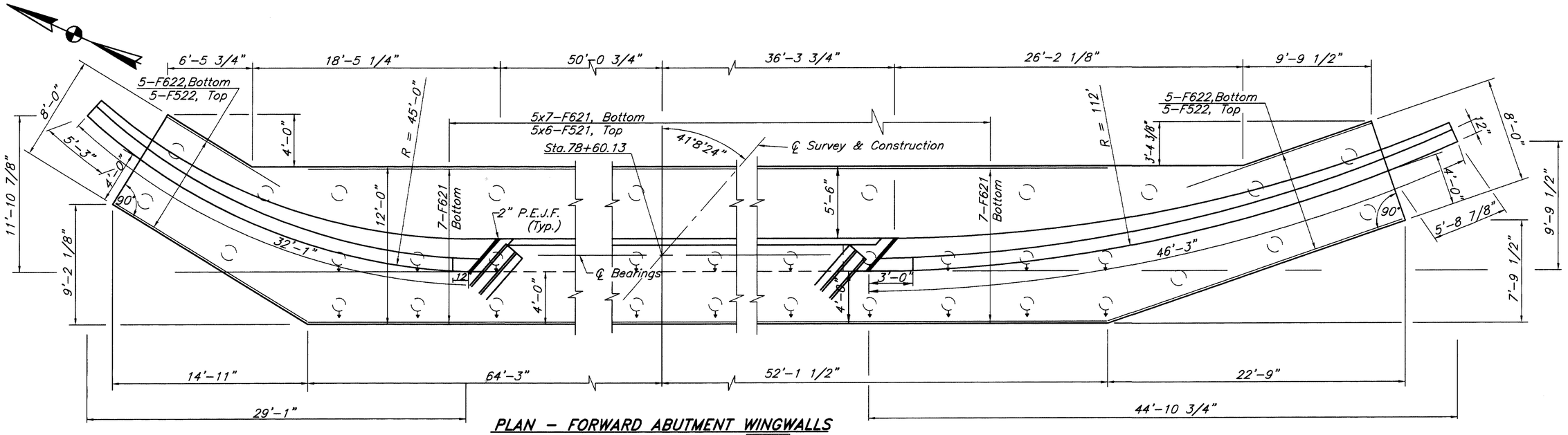
STRUCTURE FILE NO. 2400308

FORWARD ABUTMENT DETAILS

Bridge No. FAY-35-0435
EXISTING RT. 35 OVER PROPOSED US 35

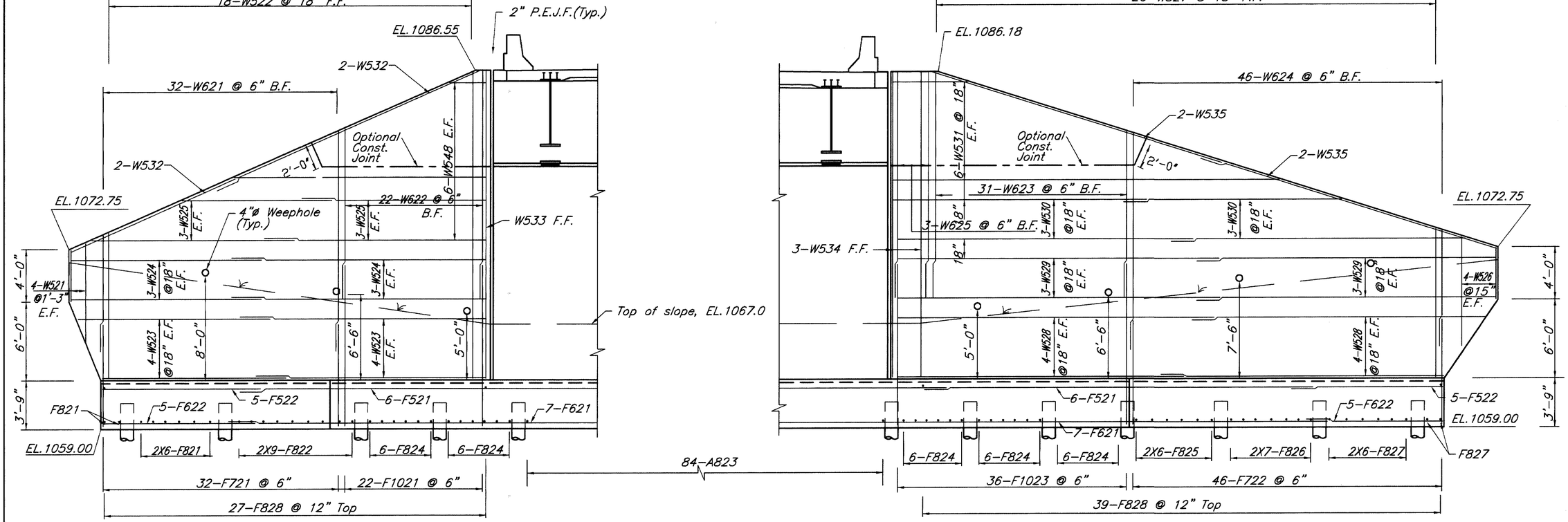
FAY-35-2.57

May 25, 1999 Q:\BRIDGE\Fay35\3595dab4.dwg



PLAN – FORWARD ABUTMENT WINGWALLS

For Pile Layout See Sheet 10/18



ELEVATION

Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

For Sections B-B & C-C See Sheet 9/18

ABBREVIATIONS
F.F. = Front Face
B.F. = Back Face
E.F. = Each Face

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

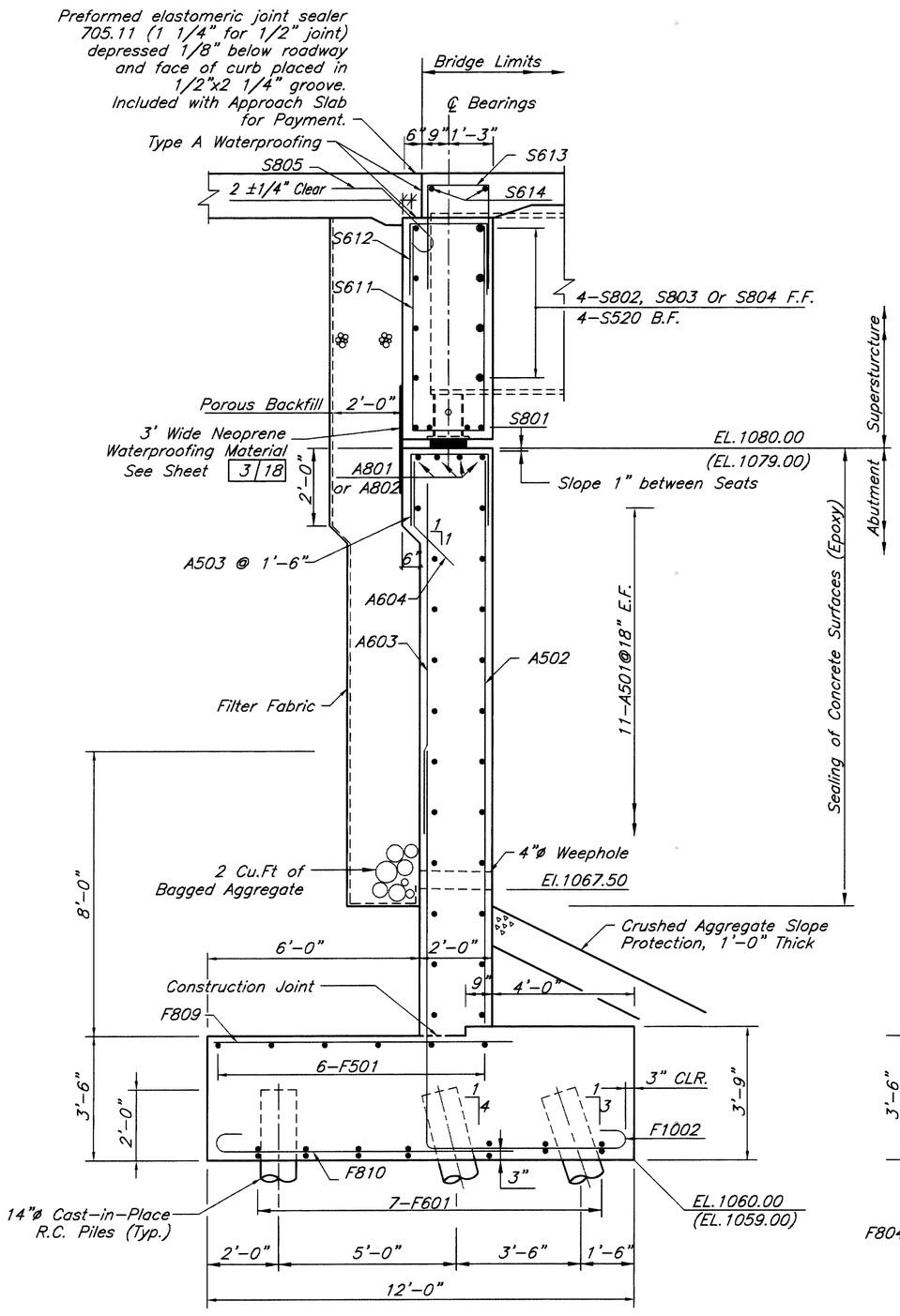
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.2400308

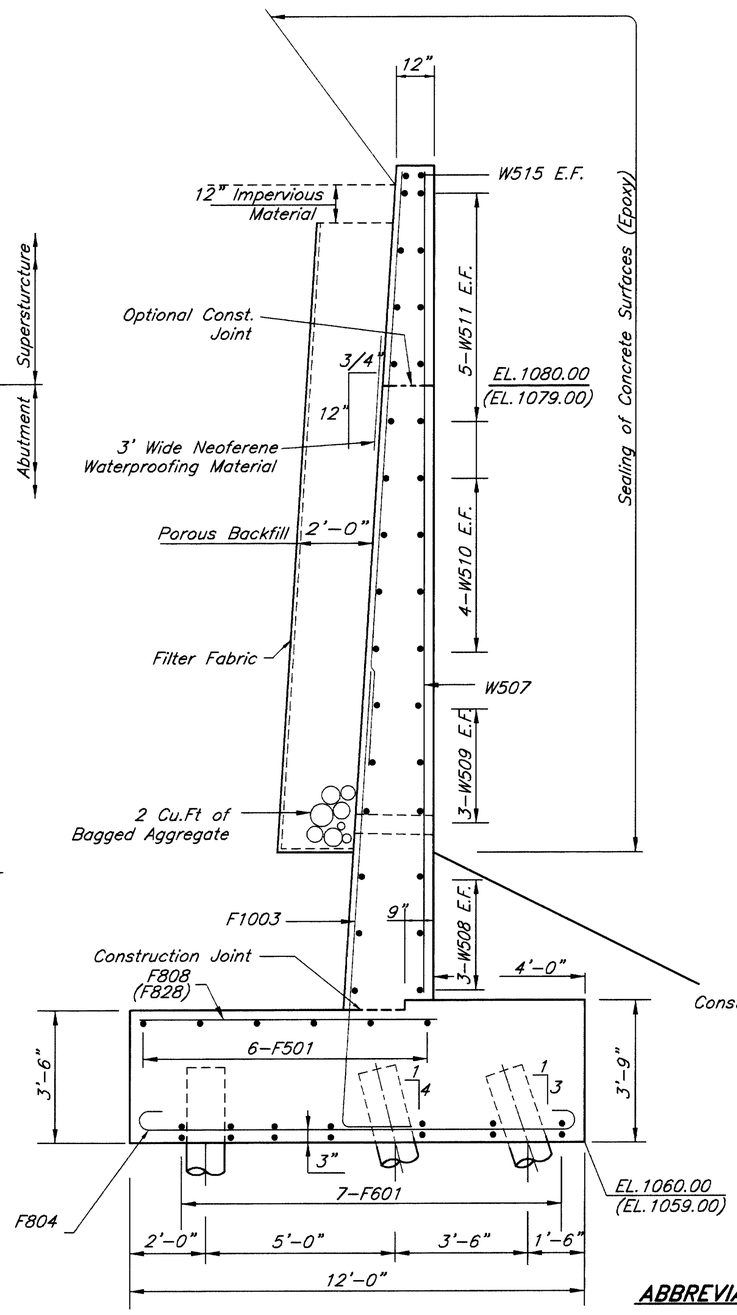
FORWARD ABUTMENT WINGWALL DETAILS

Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57



SECTION A-A



SECTION B-B

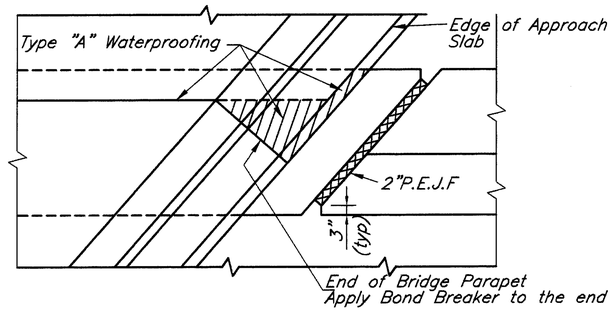
POROUS BACKFILL:

POROUS BACKFILL WITH FILTER FABRIC, 2 FEET THICK SHALL EXTEND UP TO THE PLANE OF THE SUBGRADE, TO ONE FOOT BELOW THE EMBANKMENT SURFACE, AND LATERALLY TO THE ENDS OF THE WING-WALLS. GEOTEXTILE FABRIC SHALL CONFORM WITH 712.09, TYPE A. TWO CUBIC FEET OF BAGGED NO. 3 AGGREGATE SHALL BE PLACED AT EACH WEEPHOLE. BAGGED AGGREGATE AND GEOTEXTILE FABRIC ARE INCLUDED WITH POROUS BACKFILL FOR PAYMENT.

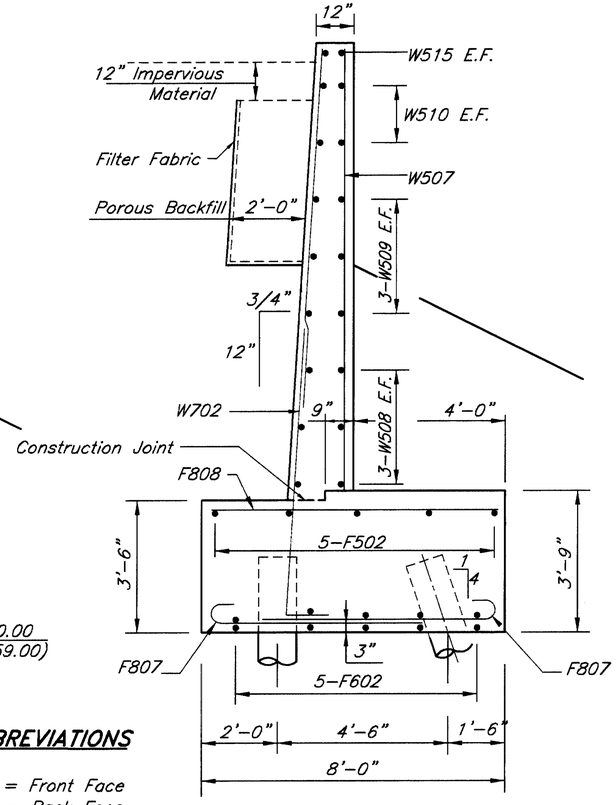
For Location of Section A-A see sheet 5 18 & 7 18

For Location of Sections B-B & C-C see sheet 6 18 & 8 18

DIAPHRAGM concrete encasing the structural member sections supported in semi-integral and integral type abutments shall be placed at least 48 hours before the actual deck concrete is placed.



DETAIL "B"
(Detail "A" Similar)



SECTION C-C

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face

Elevations in ( ) are for Forward Abutment.

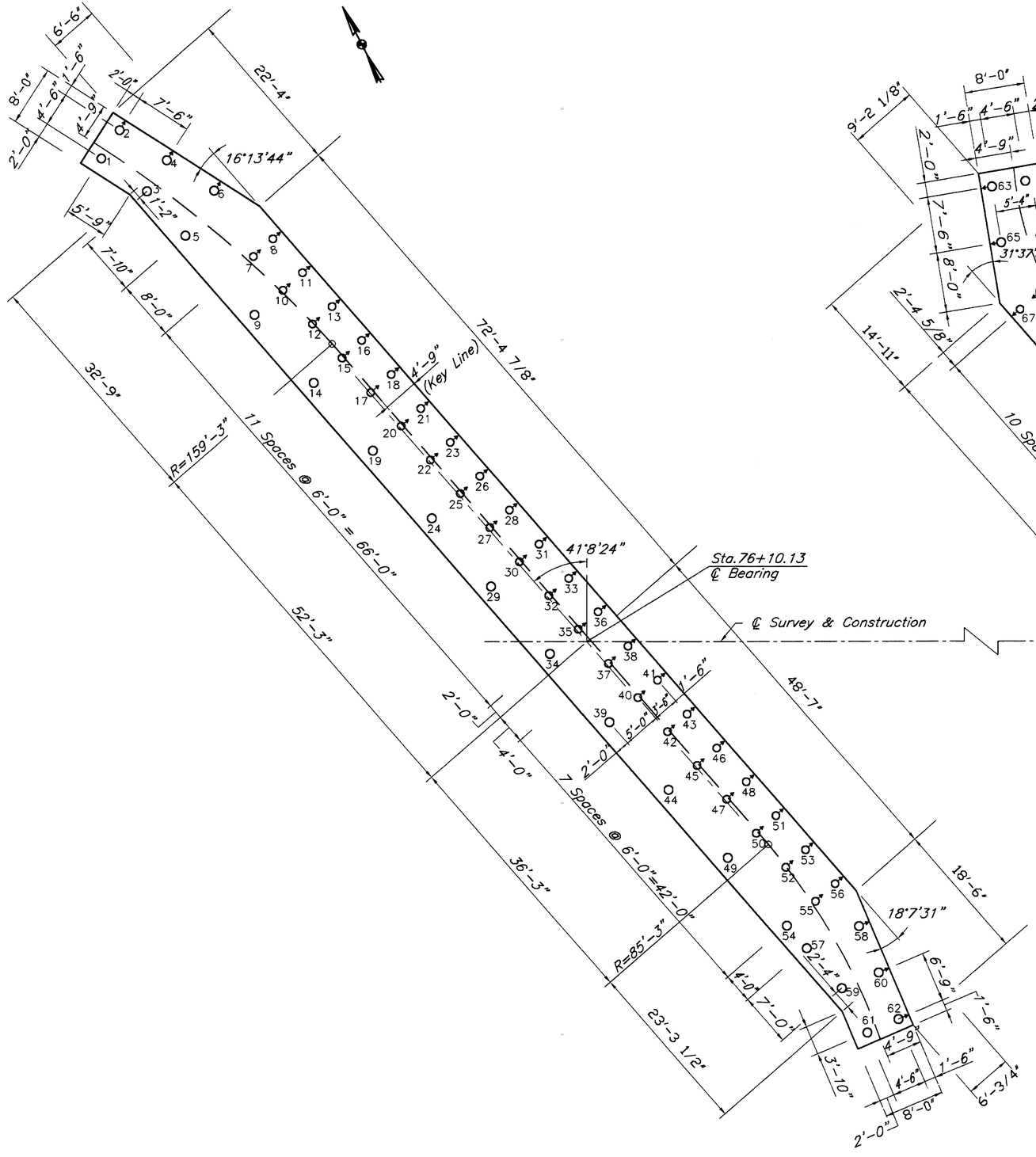
DESIGN AGENCY: ERIKSSON ENGINEERING LTD. 1523 CHESAPEAKE AVENUE COLUMBUS, OH 43212
DESIGNED: RT | DRAWN: RT | REVIEWED: CEE | DATE: 2-24-99
CHECKED: SP/VK | REVISED: | STRUCTURE FILE NO.2400308

ABUTMENT DETAILS
Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US.35

FAY-35-2.57

9 18

445
522



PILE LAYOUT & FOOTING PLANS

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.2400308

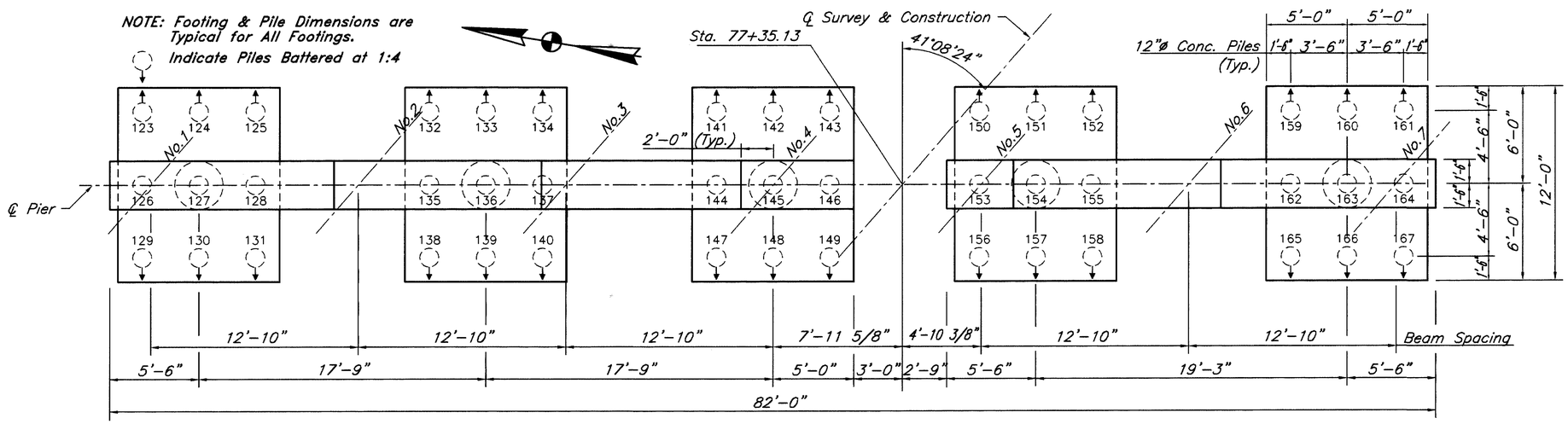
ABUTMENT PILE LAYOUT
Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57

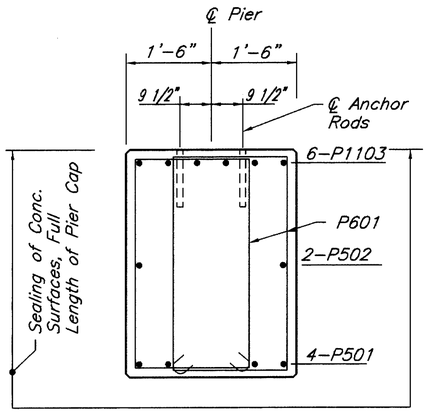
10 | 18

446
522

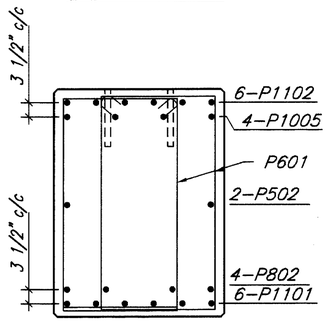
NOTE: Footing & Pile Dimensions are Typical for All Footings.
Indicate Piles Battered at 1:4



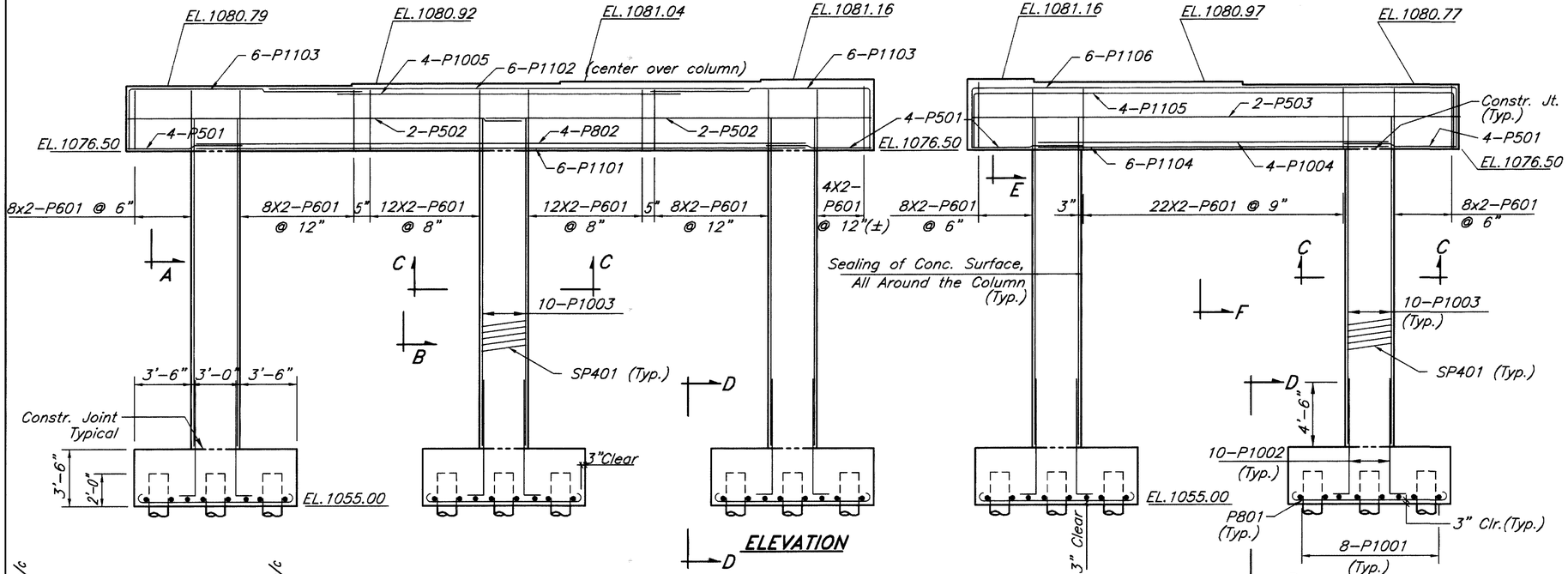
PLAN



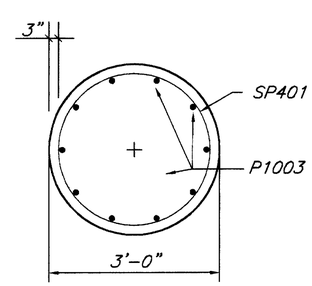
SECTION A-A



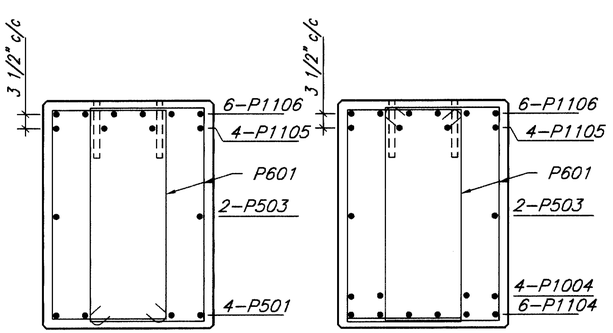
SECTION B-B



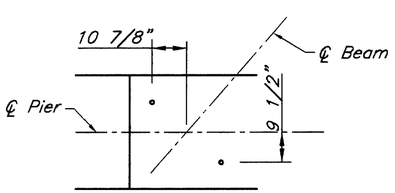
ELEVATION



SECTION C-C



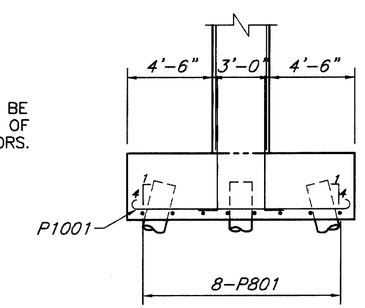
SECTIONE-E SECTION F-F



BEARING ANCHORS LAYOUT

BRIDGE SEAT REINFORCING:

REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT SHALL BE ACCURATELY PLACED TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE-SETTING OF BEARING ANCHORS.



VIEW D-D

DESIGN AGENCY: ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

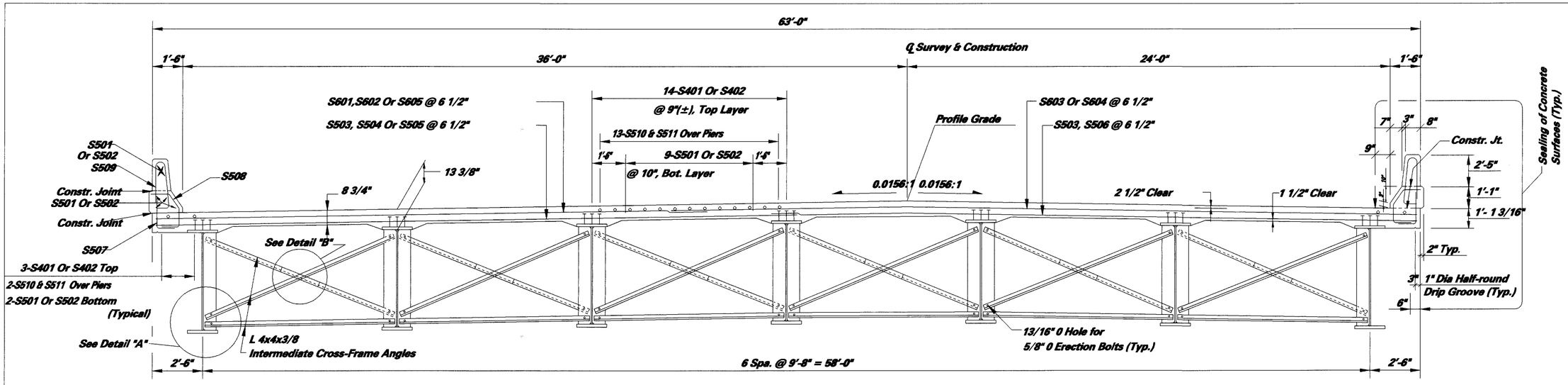
DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.2400308

PIER DETAILS
Bridge No. FAY-35-0435
EXISTING S.R.35 OVER PROPOSED U.S.35

FAY-35-2.57

11 | 18

447
522



TRANSVERSE SECTION

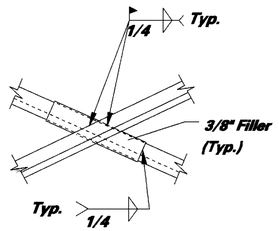
Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

** 84-S510 and 84-S511 required over pier

For Notes See Sheet 13 /18

A = 11-S507, S508 & S509 @ 1'-5"

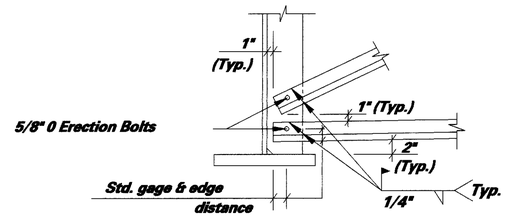
B = 6-S507, S508 & S509 @ 1'-5"



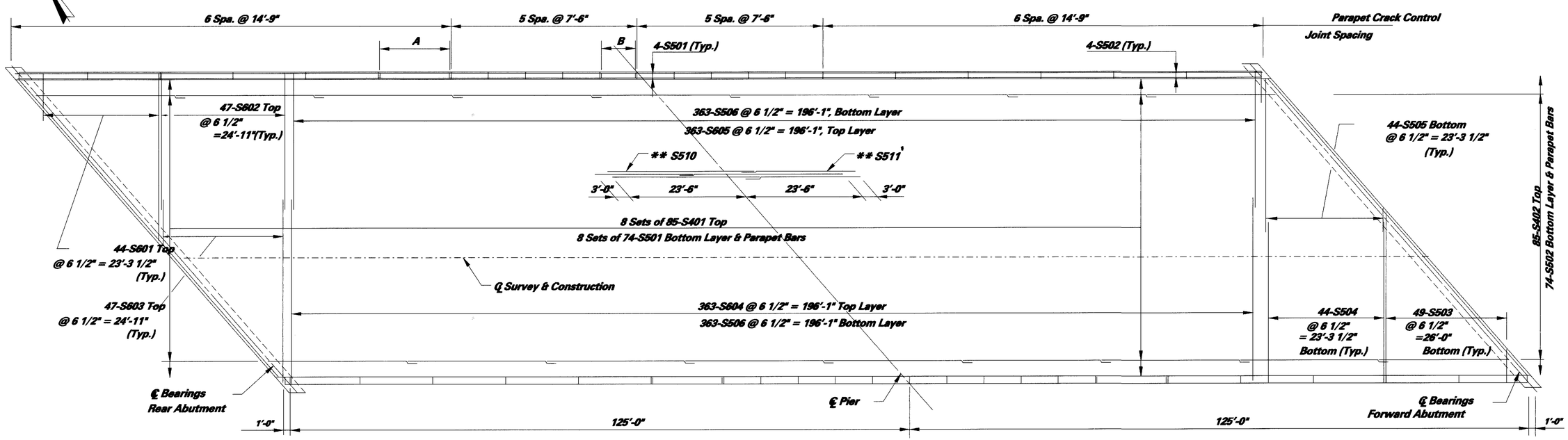
DETAIL "B"

ERECTION BOLTS: THE HOLE DIAMETER IN THE CROSS FRAMES AND GIRDER STIFFENERS SHALL BE 3/16" LARGER THAN THE DIAMETER OF THE ERECTION BOLTS. UNLESS REPLACED BY PERMANENT HIGH STRENGTH BOLTS, ERECTION BOLTS SHALL REMAIN IN PLACE. LOCK WASHERS SHALL BE FURNISHED FOR OTHER THAN FULLY TORQUED HIGH STRENGTH ERECTION BOLTS. BOLTS SHALL BE FURNISHED AS PART OF ITEM 842.

IN LIEU OF ERECTION BOLTS AND AT THE OPTION OF THE CONTRACTOR, ALTERNATIVE MEANS OF TEMPORARY BRACING MAY BE USED SUBJECT TO THE APPROVAL OF THE DIRECTOR (501.06).



DETAIL "A"



DECK REINFORCING PLAN

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURAL FILE NO. 2400308	
SP/VK			

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED U.S.35

FAY-35-2.57

12 / 18

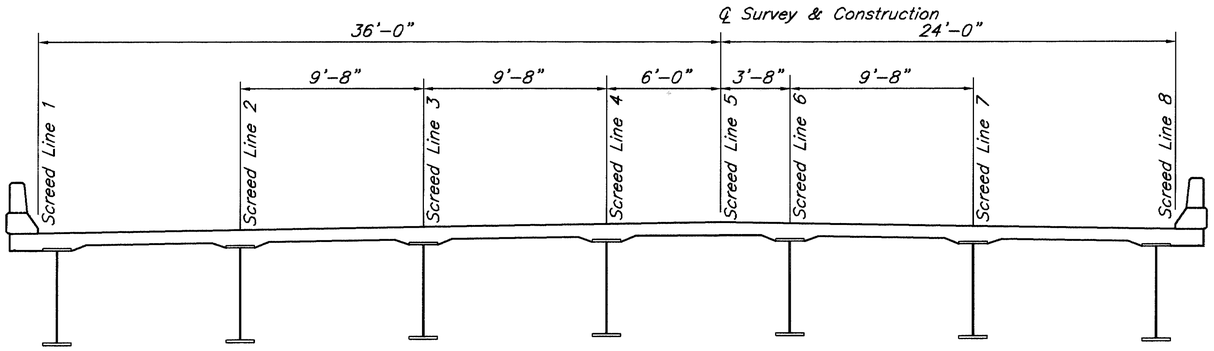
448
522

SCREED ELEVATION TABLE

		SCREED LINE 1		SCREED LINE 2		SCREED LINE 3		SCREED LINE 4		SCREED LINE 5		SCREED LINE 6		SCREED LINE 7		SCREED LINE 8	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	75+78.68	1087.26	75+88.00	1087.45	75+96.44	1087.63	76+04.89	1087.80	76+10.13	1087.91	76+13.33	1087.86	76+21.78	1087.72	76+31.10	1087.57
	0.20 L	76+03.68	1087.43	76+13.00	1087.61	76+21.44	1087.77	76+29.89	1087.94	76+35.13	1088.03	76+38.33	1087.98	76+46.78	1087.83	76+56.10	1087.67
	0.40 L	76+28.68	1087.50	76+38.00	1087.68	76+46.44	1087.83	76+54.89	1087.98	76+60.13	1088.07	76+63.33	1088.01	76+71.78	1087.85	76+81.10	1087.68
	0.60 L	76+53.68	1087.49	76+63.00	1087.65	76+71.44	1087.79	76+79.89	1087.93	76+85.13	1088.03	76+88.33	1087.95	76+96.78	1087.78	77+06.10	1087.60
	0.80 L	76+78.68	1087.40	76+88.00	1087.55	76+96.44	1087.69	77+04.89	1087.81	77+10.13	1087.89	77+13.33	1087.82	77+21.78	1087.64	77+31.10	1087.44
Span No.2	0.00 L	77+03.68	1087.31	77+13.00	1087.45	77+21.44	1087.57	77+29.89	1087.69	77+35.13	1087.76	77+38.33	1087.69	77+46.78	1087.50	77+56.10	1087.28
	0.20 L	77+28.68	1087.26	77+38.00	1087.39	77+46.44	1087.50	77+54.89	1087.60	77+60.13	1087.67	77+63.33	1087.59	77+71.78	1087.39	77+81.10	1087.16
	0.40 L	77+53.68	1087.21	77+63.00	1087.32	77+71.44	1087.42	77+79.89	1087.53	77+85.13	1087.57	77+88.33	1087.49	77+96.78	1087.28	78+06.10	1087.04
	0.60 L	77+78.68	1087.08	77+88.00	1087.18	77+96.44	1087.27	78+04.89	1087.36	78+10.13	1087.41	78+13.33	1087.32	78+21.78	1087.10	78+31.10	1086.85
	0.80 L	78+03.68	1086.87	78+13.00	1086.95	78+21.44	1087.03	78+29.89	1087.10	78+35.13	1087.14	78+38.33	1087.06	78+46.78	1086.82	78+56.10	1086.56
	1.00 L	78+28.68	1086.55	78+38.00	1086.63	78+46.44	1086.70	78+54.89	1086.76	78+60.13	1086.80	78+63.33	1087.70	78+71.78	1086.46	78+81.10	1086.18

SCREED ELEVATIONS are for the concrete surface before the concrete is placed. Allowance has been made for the anticipated deflection due to deck concrete.

L = Span Length



SCREED LINE LOCATIONS

THE DISTANCE SHOWN FROM THE TOP OF THE DECK SLAB TO THE BOTTOM OF THE TOP FLANGE IS THE THEORETICAL DESIGN DIMENSION. THE QUANTITY OF DECK CONCRETE TO BE PAID FOR SHALL BE BASED UPON THIS DIMENSION, EVEN THOUGH DEVIATION FROM IT MAY BE NECESSARY BECAUSE THE TOP FLANGE OF THE GIRDER MAY NOT HAVE THE EXACT CAMBER OR CONFORMATION REQUIRED TO PLACE IT PARALLEL TO THE FINISHED GRADE. DEDUCTION SHALL BE MADE FOR VOLUME OF ENCASED STEEL PLATES AS PER 511.18.

CONCRETE DECK HAUNCH WIDTH

A HAUNCH WIDTH OF 9 INCHES SHALL BE USED FOR COMPUTING QUANTITY OF CONCRETE. HOWEVER, THE HAUNCH WIDTH MAY VARY BETWEEN 6 AND 12 INCHES.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

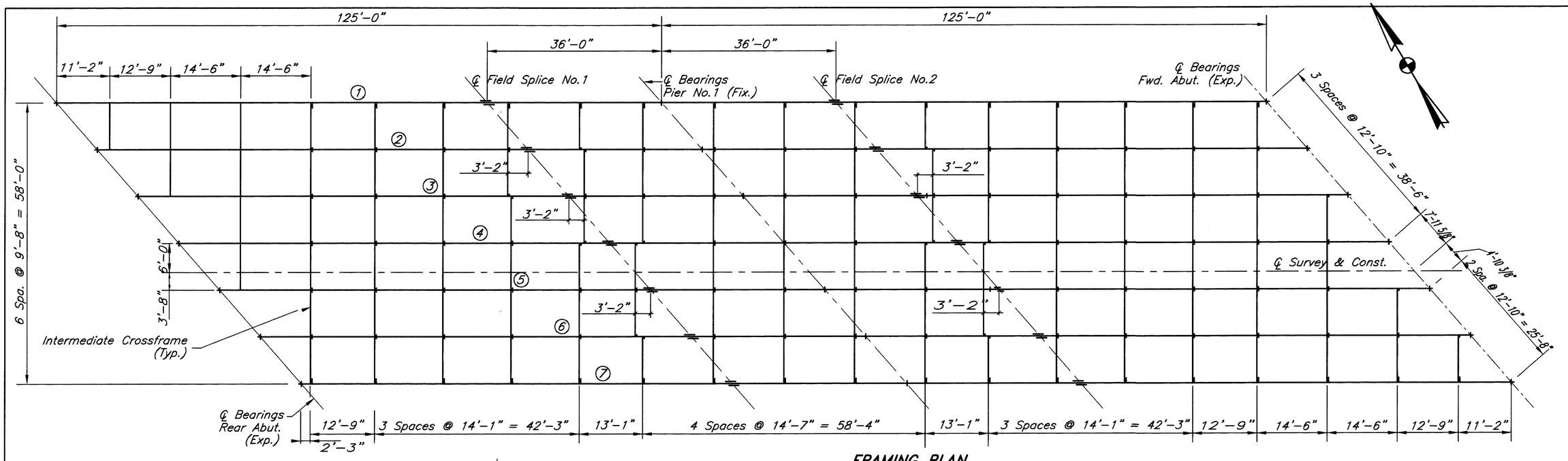
DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99 | STRUCTURE FILE NO.2400308
CHECKED SP/VK | REVISED

SCREED ELEVATIONS
Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED U.S.35

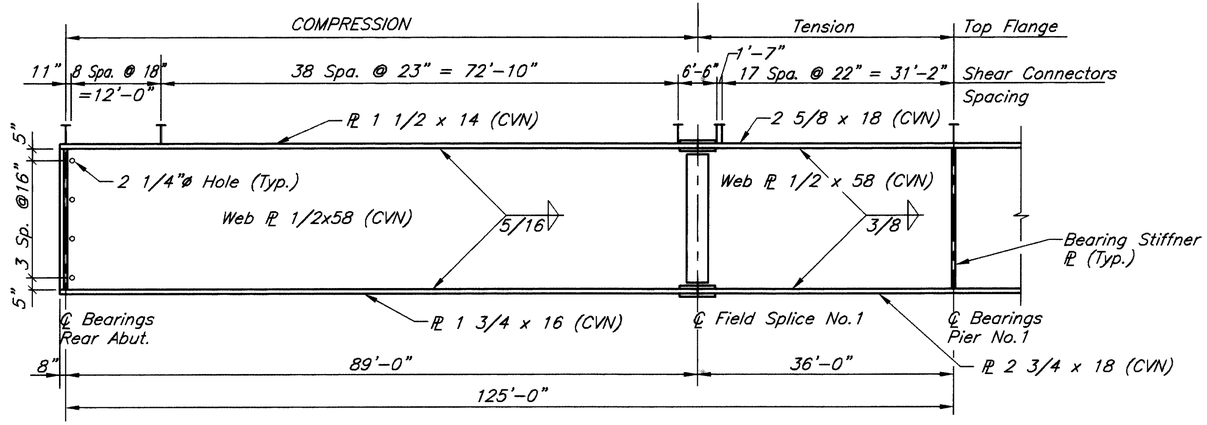
FAY-35-2.57

13 / 18

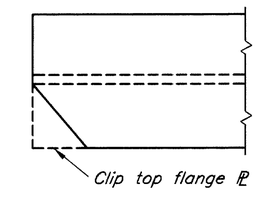
449
522



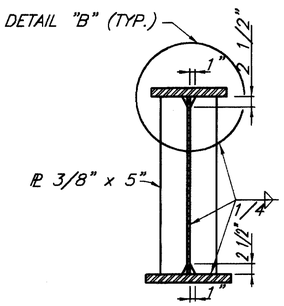
FRAMING PLAN



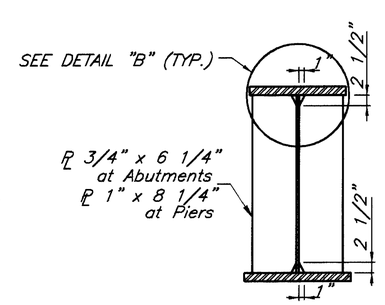
HALF-GIRDER ELEVATION



TOP FLANGE PL DETAIL



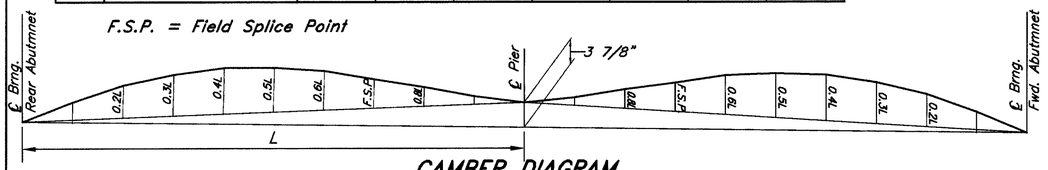
CROSSFRAME CONNECTION PLATE DETAIL



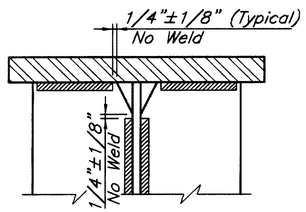
BEARING STIFFENER DETAIL

DEFLECTION AND CAMBER								
	Points	Span No.1						
		.20	.30	.40	.50	.60	F.S.P.	.80
ALL GIRDERS	Deflection due to weight of steel	5/16	3/8	7/16	3/8	5/16	3/16	1/8
	Deflection due to remaining dead load	1 1/8	1 7/16	1 1/2	1 7/16	1 3/16	13/16	7/16
	Adjustment for Vertical curve	3/4	1	1 3/16	1 3/16	1 3/16	1	3/4
	Net Total Camber	2 3/16	2 13/16	3 1/8	3	2 11/16	2	1 5/16

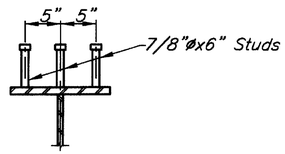
F.S.P. = Field Splice Point



CAMBER DIAGRAM



DETAIL "B"



SHEAR CONNECTOR DETAIL

NOTES :

WELDED ATTACHMENT *of supports for concrete deck finishing machine may be made to areas of the flanges designated "Compression". Attachments shall not be made to areas designated "Tension". Fillet welds to compression flanges shall be not closer than 1" from edge of the flange, be not more than 2" long and be not smaller than the minimum size required by AASHTO.*

For additional notes see sheet 15/18

For details of bearing stiffeners at abutments see sheet 16/18

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.2400308

SUPERSTRUCTURE DETAILS

BRIDGE NO. FAY-35-0435
EXISTING RT. 35 OVER PROPOSED US 35

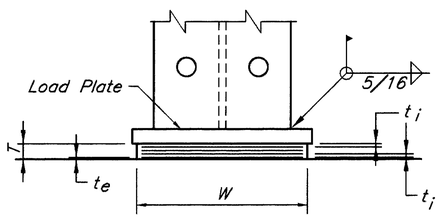
FAY-35-2.57

LAMINATED ELASTOMERIC BEARINGS

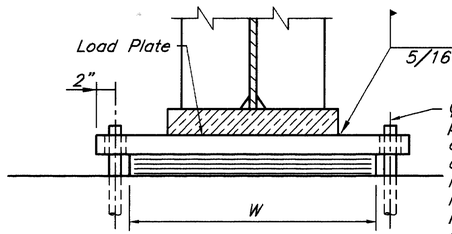
LOCATION	DIMENSIONS (Inch)					N	STEEL LOAD PLATE	REACTIONS		MAXIMUM DESIGN LOAD
	L	W	t_i	t_e	T			DL	LL	
Abutments Expansion Brngs.	12	18	0.281	0.201	1 5/8	4	13" x 19" x 1 1/2"	132.3 k	60.5 k	192.8 k
Pier Fixed Brngs.	18	26	0.414	0.296	2 1/4	4	19" x 33" x 2"	356.3 k	112.8 k	469.1 k

t_i = Thickness of Internal Layer
t_e = Thickness of External Layer
T = Total Thickness of Elastomeric Bearing

N = No. of Steel Laminates
Internal Steel Laminate Thickness = 0.0747"
Durometer of Elastomer = 50 Durometer



LAMINATED ELASTOMERIC EXPANSION BEARING



℄ 1 3/4" hole in steel load plate for 1 1/4" ø x 1'–7" anchor rod, galvanized according to 711.02. Install anchor rod per Item 510. Include dowel holes and anchor rods with Item 516, Laminated Elastomeric Bearings for payment.

LAMINATED ELASTOMERIC FIXED BEARING

LOAD PLATE

The steel load plate shall be the same material as the attached structural steel and be similarly cleaned and coated. Surface preparation and priming shall be done in the shop and be included in the price bid for the bearings. Field coats shall be included in the price bid for painting main structural steel.

The steel load plate shall be bonded by vulcanization to the elastomer during the molding process.

Welding of the load plate to the superstructure shall be controlled so that the plate temperature at the elastomer bonded surface shall not exceed 300° F as determined by the use of pyrometric sticks or other temperature monitoring devices.

BASIS OF PAYMENT

The unit bid price shall include all materials, labor and incidentals necessary to furnish and install laminated elastomeric bearings either fixed or expansion. Payment will be made at the contract price for Item 516, each, Laminated Elastomeric Bearings.

ELASTOMERIC BEARINGS

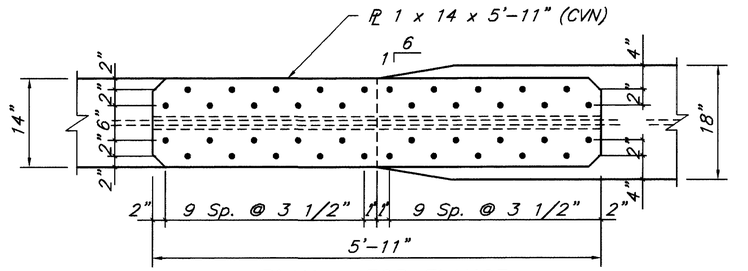
Elastomeric bearings shall comply with Item 516 and articles 18.2.5 through 18.2.8, Section 18, Bearing Devices, Division II, Construction, of the AASHTO Specifications for Highway Bridges. Bearings shall be Grade 3, 50–durometer elastomer and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the price bid for the bearings.

HIGH STRENGTH BOLTS

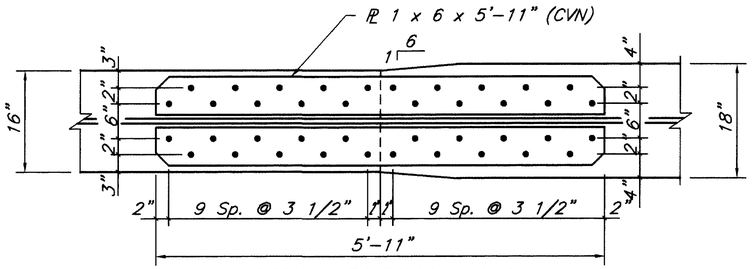
High strength bolts shall be 1" diameter A325, galvanized, unless otherwise noted.

CHARPY V–NOTCH TOUGHNESS REQUIREMENT

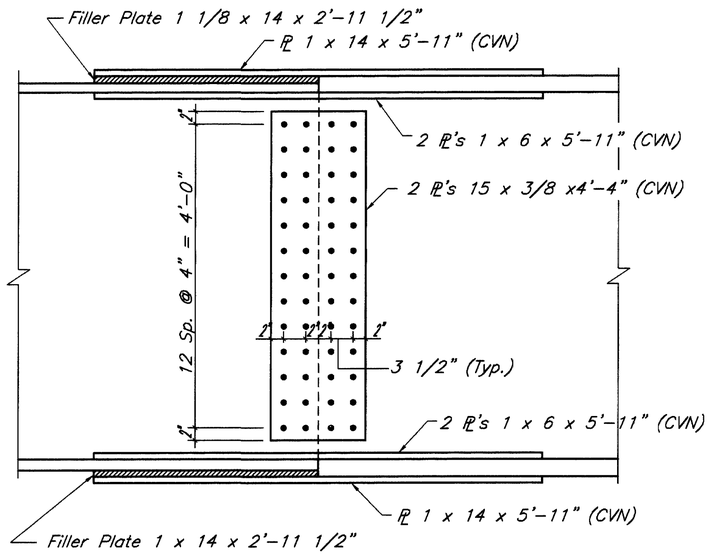
Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in 711.01 of CMS.



PLAN – TOP FLANGE



PLAN – BOTTOM FLANGE



TYPICAL FIELD SPLICE DETAILS

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

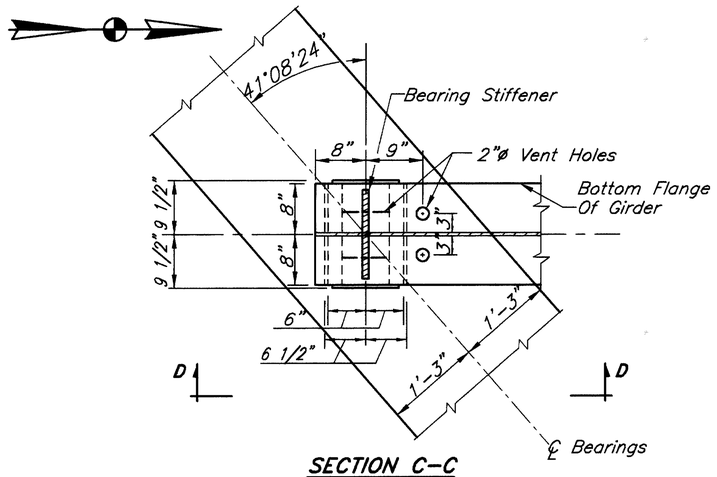
DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO. 2400308
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

BEARING & SPLICE DETAILS
Bridge No. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

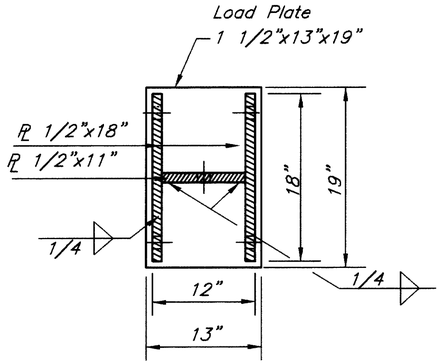
FAY-35-2.57

15 / 18

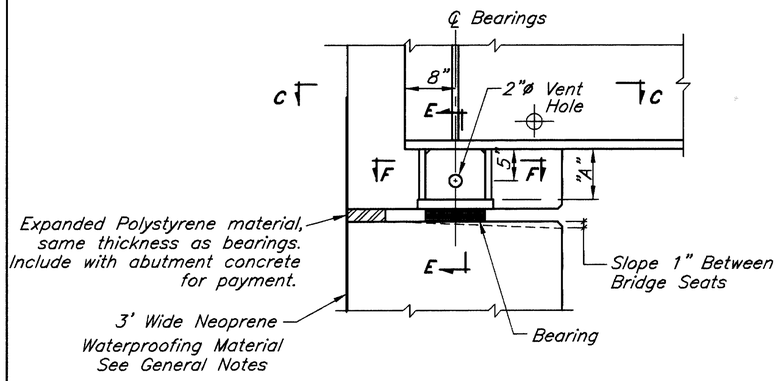
451
522



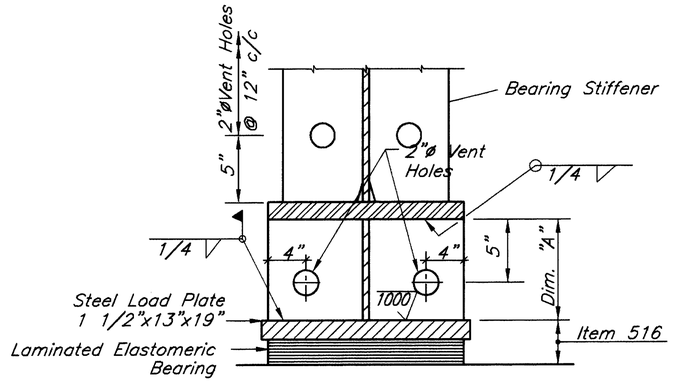
SECTION C-C



SECTION F-F



SECTION D-D



SECTION E-E

BEARING SUPPORT -DIM. "A"							
GIRDER NOS.	**1**	**2**	**3**	**4**	**5**	**6**	**7**
Rear Abutment	11 1/16"	1'-1 3/16"	1'-3 5/16"	1'-5 3/8"	1'-6 1/16"	1'-4 7/16"	1'-2 3/4"
Forward Abutment	1'-2 1/2"	1'-3 3/8"	1'-4 1/16"	1'-4 15/16"	1'-4 3/16"	1'-1 3/16"	10 3/16"

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO. 2400308	
VK			

BEARING AND JOINT SEAL DETAILS
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US.35

FAY-35-2.57

16 / 18

452
522

REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				ABUTMENTS					
A501	132	30–0	4130	STR					
A502	120	15–11	1992	STR					
A503	120	5–5	678	2	2–2	1–9			
A603	280	10–0	4206	STR					
A604	120	3–0	541	4	1–7	1–6			
A801	16	30–0	1282	STR					
A802	8	33–6	716	STR					
			13545	LBS					
			REAR ABUTMENT FOOTING						
F501	30	30–0	939	STR					Bend in Field
F502	10	12–0	125	STR					as Required
F601	35	30–0	1577	STR					Bend in Field
F602	10	15–0	225	STR					as Required
F801	14	6–6	243	8	5–7				
F802	12	7–4	235	8	6–5				
F803	12	8–3	264	8	7–4				
F804	30	13–4	1068	12	11–6				
F805	16	8–0	342	8	7–1				
F806	14	6–11	259	8	6–0				
F807	16	6–8	285	8	5–9				
F808	80	7–8	1638	STR					
F809	89	8–6	2020	STR					
F810	88	9–3	2173	8	8–4				
F1002	140	18–11	11396	10	5–7	12–3			
			22789	LBS					
			REAR ABUTMENT LEFT WINGWALL						
W501	8	4–0 to	52	STR					2 Sets of 4,
		8–6							Vary by 1'–6"
W502	20	9–9 to	342	STR					Vary by 8 3/8"
		23–0							
W503	12	17–0 to	222	STR					4 Sets of 3,
		18–6							Vary by 0'–9"
W504	12	19–6	244	STR					
W505	12	15–0 to	207	STR					4 Sets of 3
		18–0							Vary by 1'–6"
W506	14	3–9 to	197	STR					2 Sets of 7,
		23–3							Vary by 3'–3"
W512	4	20–6	86	STR					
W513	1	23–8	25	STR					
W601	37	8–9 to	695	STR					Vary by 2 1/2"
		16–3							
W602	22	13–2 to	507	STR					Vary by 2 1/2"±
		17–6							
F701	37	8–10	668	11	1–2	7–10			In Footing
F1001	22	13–9	1302	11	1–10	12–3			In Footing
			4547	LBS					

Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
			REAR ABUTMENT RIGHT WINGWALL						
W506	8	3–10 to	48	STR					2 Sets of 4,
		7–7							Vary by 1'–3"
W507	29	9–3 to	492	STR					Vary by 6"
		23–3							
W508	12	24–10 to	323	STR					4 Sets of 3,
		26–10							Vary by 1'–0"
W509	12	27–4	342	STR					
W510	16	25–9 to	369	STR					4 Sets of 4,
		18–5							Vary by 2'–5 1/2"
W511	12	28–0 to	207	STR					2 Sets of 6,
		5–0							Vary by 4'–7"±
W514	3	23–4	73	STR					
W515	4	26–3	110	STR					
W516	2	4–6	9	STR					Bend in Field
W603	38	11–10 to	851	STR					Vary by 2"
		18–0							
W604	48	7–3 to	805	STR					Vary by 2"
		15–1							
W605	3	17–0	77	STR					
F702	48	8–10	867	11	1–2	7–10			In Footing
F1003	43	13–9	2544	11	1–10	12–3			In Footing
			7117	LBS					
			FORWARD ABUTMENT FOOTING						
F521	30	30–0	939	STR					Bend in Field
F522	10	8–3	86	STR					as Required
F621	35	30–0	1577	STR					Bend in Field
F622	10	10–3	154	STR					as Required
F821	16	6–7	281	8	5–8				
F822	18	8–4	400	8	7–5				
F823	84	9–3	2075	8	8–4				
F824	30	13–4	1068	12	11–6				
F825	12	8–0	256	8	7–1				
F826	14	7–8	287	8	6–9				
F827	16	6–7	281	8	5–8				
F828	66	7–8	1351	STR					
F829	89	8–6	2020	STR					
F1022	140	18–11	11396	10	5–7	12–3			
			22171	LBS					

ALL BARS SHALL BE EPOXY COATED

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
DRAWN SP
REVISED
DESIGNED SP
CHECKED VK

STRUCTURE FILE NO.2400308

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0435
EXISTING RT.35 OVER PROPOSED US 35

FAY-35-2.57

17 / 18

453
522

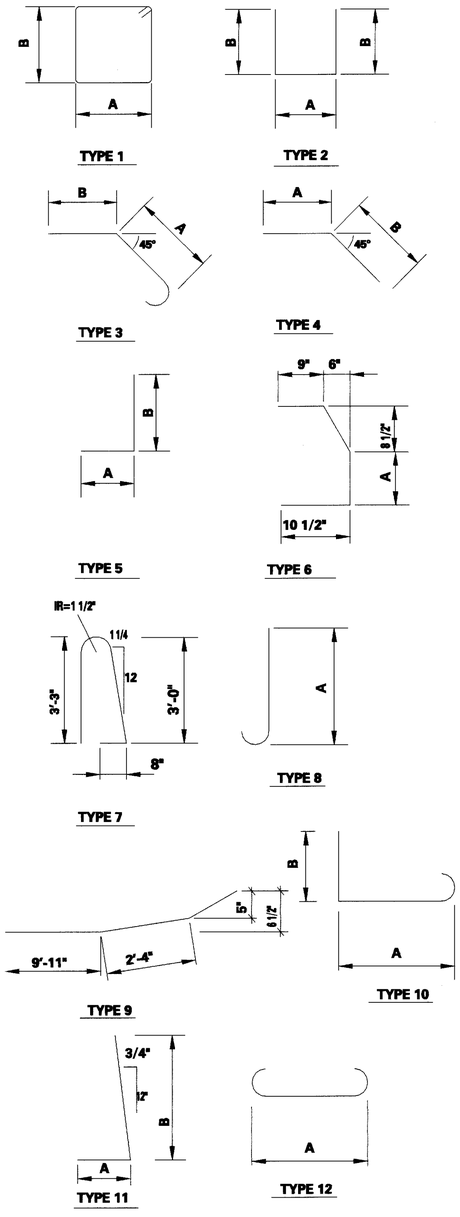
REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
		FORWARD ABUTMENT (LEFT WINGWALL)							
W521	8	3-11 to	57	STR					2 Sets of 4,
		8-5							Vary by 1'-6"
W522	18	12-0 to	332	STR					Vary by 8"
		23-4							
W523	16	14-6 to	263	STR					4 Sets of 4,
		17-0							Vary by 1'-3"
W524	12	17-9	222	STR					
W525	12	14-0 to	194	STR					4 Sets of 3,
		17-0							Vary by 1'-6"
W532	4	18-9	78	STR					
W533	1	23-7	25	STR					
W548	12	3-6 to	148	STR					2 Sets of 6,
		20-2							Vary by 3'-4"
W621	32	10-5 to	671	STR					Vary by 2 3/4" (±)
		17-6							
W622	22	13-0 to	509	STR					Vary by 2 3/4" (±)
		17-10							
F721	32	8-10	578	11	1-2	7-10			
F1021	22	13-9	1302	11	1-10	12-3			
			4373	LBS					
		FORWARD ABUTMENT (RIGHT WINGWALL)							
W526	8	3-11 to	50	STR					2 Sets of 4,
		8-2							Vary by 1'-5"
W527	26	11-5 to	467	STR					Vary by 5 1/2" (±)
		23-0							
W528	16	22-7 to	396	STR					4 Sets of 4,
		24-10							Vary by 9"
W529	12	25-1	314	STR					
W530	12	19-7 to	273	STR					4 Sets of 3,
		24-0							Vary by 2'-2 1/2"(±)
W531	12	28-0 to	213	STR					2 Sets of 6,
		6-0							Vary by 4'-5"(±)
W534	3	23-3	73	STR					
W535	4	25-0	104	STR					Bend in Field
W623	31	13-0 to	722	STR					Vary by 2"
		18-0							
W624	46	9-3 to	907	STR					Vary by 2"(±)
		17-0							
W625	3	18-0	81	STR					
F722	46	8-10	831	11	1-2	7-10			
F1023	36	13-9	2130	11	1-10	12-3			
			6561	LBS					
		PIER							
P501	16	6-9	113	STR					
P502	4	23-10	99	STR					
P503	2	29-11	62	STR					
P601	180	12-11	3492	1	2-3	3-11			
P801	40	11-4	1210	12	9-6				
P802	4	31-2	333	STR					

Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
P1001	40	14-4	2467	12	11-6				
P1002	50	9-3	1980	5	1-10	7-9			
P1003	50	21-0	4518	STR					
P1004	4	22-3	383	STR					
P1005	4	21-8	373	STR					
P1101	6	38-6	1227	STR					
P1102	6	30-0	956	STR					
P1103	12	17-5	1110	5	14-0	3-9			
P1104	12	22-0	1403	STR					
P1105	4	33-8	715	2	28-9	3-2			
P1106	6	36-7	1166	2	29-9	3-9			
SP401	5		1709	B.T.					
			23326	LBS					
		SUPERSTRUCTURE (DIAPHRAGMS, BOTH ABUTMENTS)							
S520	24	30-0	751	STR					
S611	160	12-0	2884	2	2-2	5-1			
S612	160	8-6	2043	2	2-2	3-4			
S613	156	7-6	1757	2	1-8	3-1			
S614	16	23-3	559	STR					
S801	32	24-4	2079	STR					
S802	16	24-0	1025	STR					
S803	8	27-0	576	STR					
S804	8	22-3	475	STR					
S805	86	5-0	1148	3	1-5	2-8			
			13287	LBS					
		SUPERSTRUCTURE (DECK)							
S401	680	30-0	13627	STR					
S402	85	25-0	1429	STR					
S501	592	30-0	18524	STR					
S502	74	27-8	2135	STR					
S503	98	6-7 to	2104	STR					2 Sets of 49,
		34-7							Vary by 7"
S504	88	32-4	2968	STR					
S505	88	7-3 to	1817	STR					2 Sets of 44,
		32-4							Vary by 7"
S506	726	32-4	24488	STR					
S507	384	2-6	1001	5	10 1/2	1-9			
S508	384	3-6	1402	6	1-1 1/2				
S509	384	7-1	2837	7					
S510	84	30-0	2628	STR					
S511	84	20-0	1752	STR					
S601	88	7-0	2693	STR					2 Sets of 44,
		33-9							Vary by 7 1/2"±
S602	94	27-8	3906	STR					
S603	94	8-3 to	3194	STR					2 Sets of 47,
		37-0							Vary by 7 1/2"
S604	363	37-4	20365	STR					Pitch 4 1/2"
S605	363	27-8	15085	STR					
			121931	LBS					

ALL BARS SHALL BE EPOXY COATED



MARK	NO.	LENGTH	WEIGHT	SHAPE	REMARKS
SP401	5	18-2	1709	BT.	
			1709	LBS.	

SP401 - Core Diameter 30" c/c

Other Details in Accordance with C.R.S.I. Standard Practice.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Four steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 3.20 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

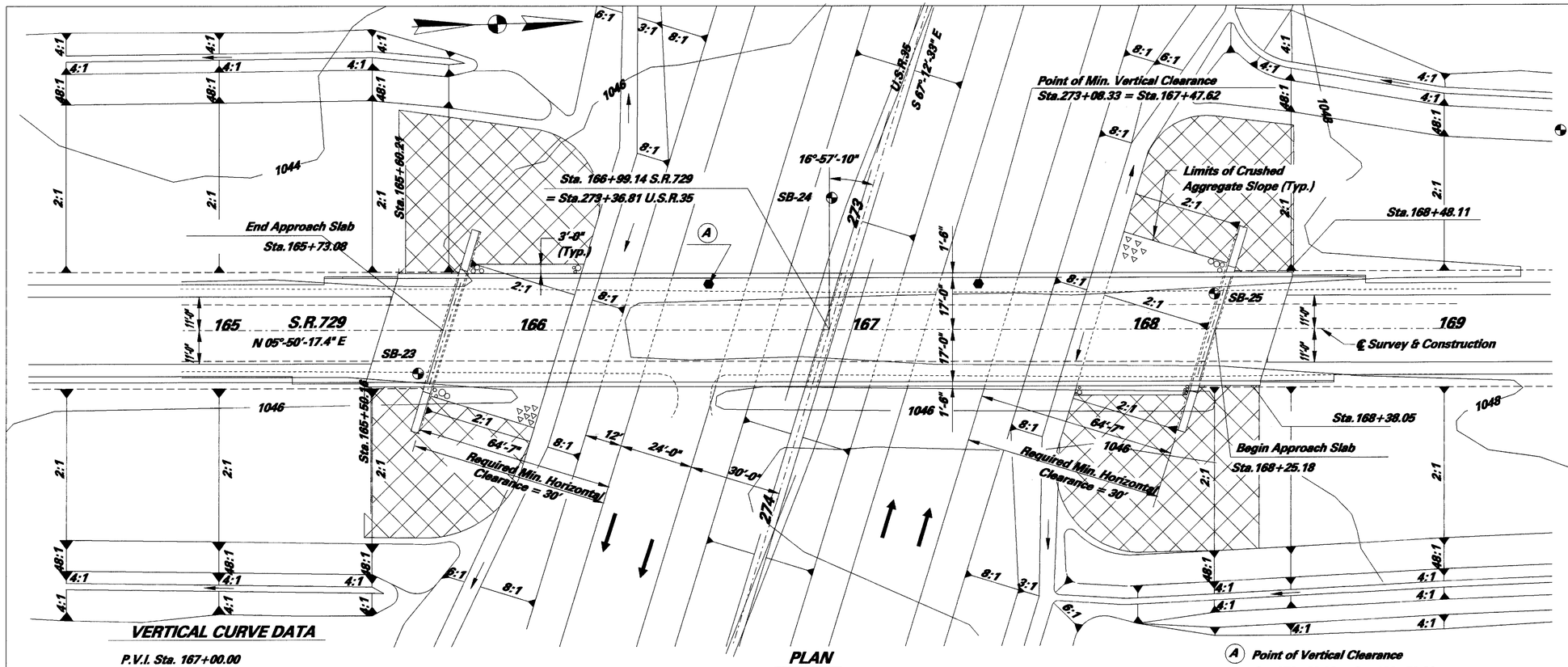
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO.2400308
DRAWN SP
REVISED
DESIGNED SP
CHECKED VK

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0435
EXISTING US.35 OVER PROPOSED US 35

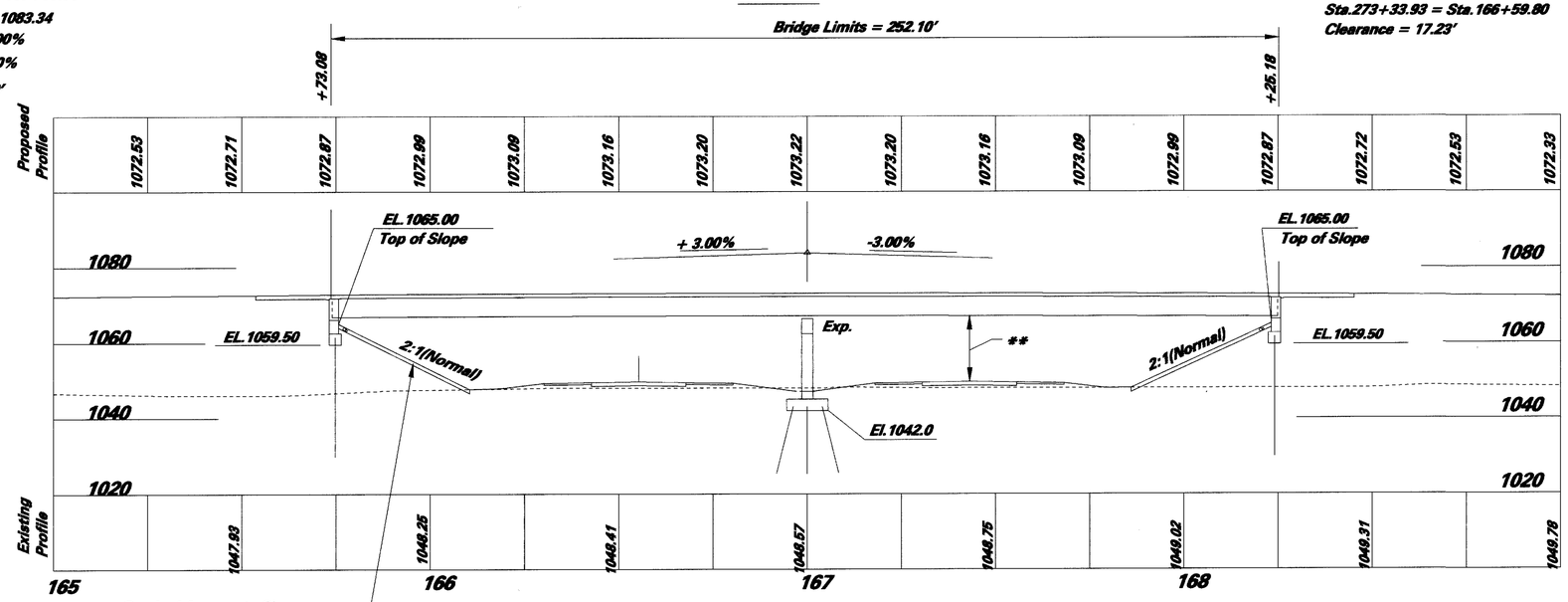
FAY-35-2.57



VERTICAL CURVE DATA

P.V.I. Sta. 167+00.00
P.V.I. EL. 1083.34
G_1 = +3.00%
G_2 = -3.00%
L = 1350'

PLAN



Crushed Aggregate Slope Protection, 1'-0" Thick (Typ.) Included with Roadway Quantities for Payment.

PROFILE

On ℄ Survey & Construction

(A) Point of Vertical Clearance
Sta.273+33.93 = Sta.166+59.80
Clearance = 17.23'

** Required Min. Vertical Clearance = 16.5'
Provided Min. Vertical Clearance = 17.12'

DESIGN TRAFFIC

1997 ADT 800
2017 ADT 1120
18 % Trucks

B.M.-TR106

Iron Pin With Cap
Sta.167+73.10, 19.8' Right
Elev. = 1046.457

PILE DATA

Location	Type	Estimated Pile Length
Abutments	12"0 Conc.	50 Ft.
Pier	12"0 Conc.	30 Ft.

PROPOSED STRUCTURE

TYPE: Continuous Plate Girders (A572, Painted) on Reinforced Concrete Substructure

SPAN: 2 Spans @ 125'-0" c/c Bearings

ROADWAY: 34'-0" Toe to Toe of Parapets.

LOADING: HS20-44(Case II) And The Alternate Military Loading

SKEW: 16°-57'-10" L.F.

WEARING SURFACE: Monolithic Concrete

APPROACH SLABS: 20' Long (Special)

ALIGNMENT: Tangent

CROWN : 0.0156:1

LATITUDE : 39°-36'-20.4"N

LONGITUDE : 83°-35'-12.82"W

USGS QUADRANGLE : Milledgeville

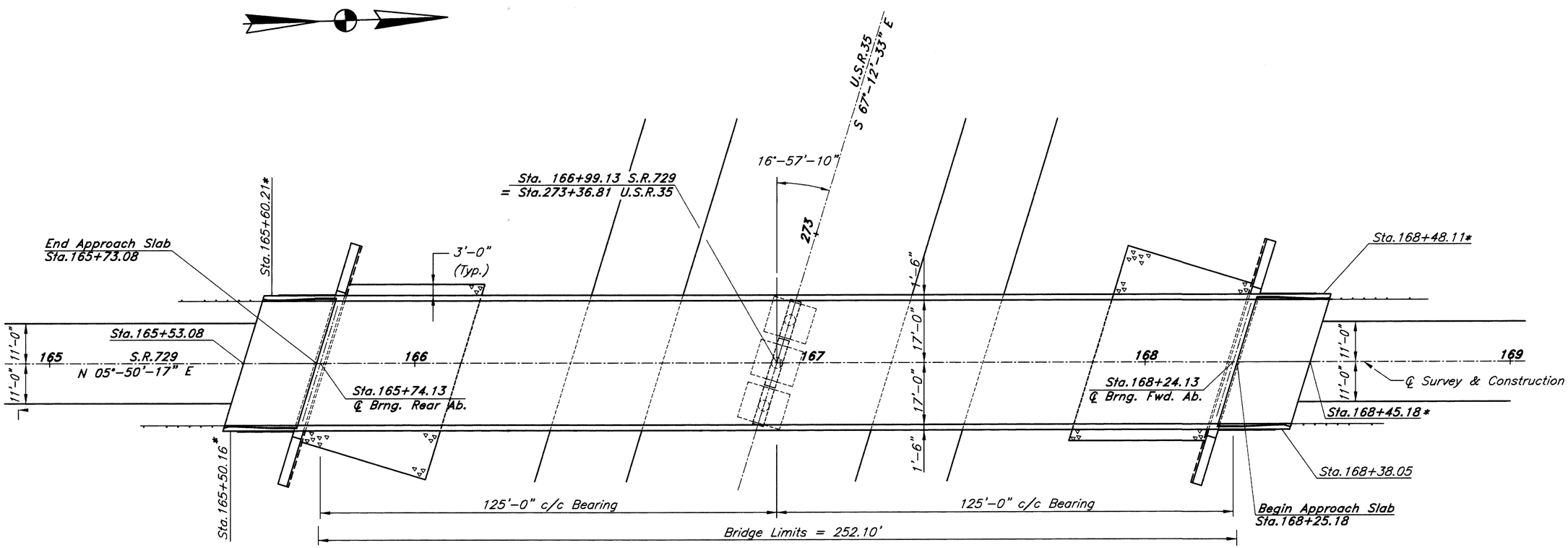
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
STRUCTURE FILE NO.
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP

FAYETTE COUNTY
STA.165+73.08
STA.168+25.18

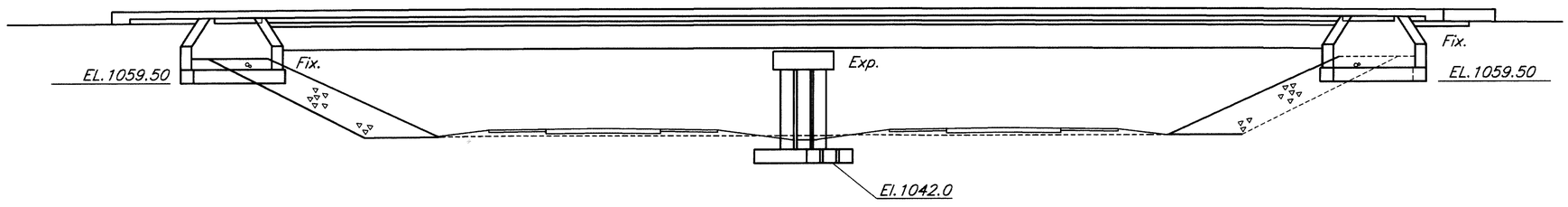
SITE PLAN
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57



GENERAL PLAN

* Station are given at ℄ Guardrail Terminal Assembly



ELEVATION

Piles not shown.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-99
REVIEWED CEE
DRAWN RT
REVISED
DESIGNED RT
CHECKED SP/VK

FAYETTE COUNTY
STA.165+73.08
STA.168+25.18

GENERAL PLAN & ELEVATION
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57

2 / 12

456
522

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844	-	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C	-	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL	-	ASTM A615, A616 OR A617 GRADE 60 UNIT STRESS 24,000 PSI
STRUCTURAL STEEL	-	ASTM A572-YIELD STRIENGTH 50,000 P.S.I.

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81	DATED 9-15-94	SHEET NO. 1. AND 2.	
BR-1	DATED 1-6-99		
GSD-1-96	DATED 2-12-97		SHEETS NO. 1 & NO. 3
ICD-1-82 M	DATED 3-20-95		

AND TO SUPPLEMENTAL SPECIFICATIONS:

816	DATED 4-21-97
842	DATED 1-6-99
844	DATED 1-6-99
863	DATED 10-12-99

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, TOP & BOTTOM MATS 2 1/2" CONCRETE COVER TOP, 1 1/2" COVER BOTTOM AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED AND HAS UNDERGONE A WAITING PERIOD OF 30 DAYS.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 98 TONS PER PILE FOR THE 12" CIP ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 84 TONS PER PILE FOR THE 12" CIP PIER PILES.

ABUTMENT PILES:
- 22 PILES 50 FEET LONG, ESTIMATED LENGTH
- 22 PILES OF ORDER LENGTH 55 FEET LONG
- 11 SPLICES

PIER PILES:
- 27 PILES 30 FEET LONG, ESTIMATED LENGTH
- 27 PILES OF ORDER LENGTH 35 FEET LONG
- 13 SPLICES

ABUTMENT BACKFILL

ABUTMENT BACKFILL SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.08 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

CONCRETE PARAPETS:

WITHIN 48 HOURS AFTER PLACEMENT OF PARAPET CONCRETE SAWCUT 1 INCH DEEP JOINTS INTO THE CONCRETE PARAPET AT LOCATIONS AS DETAILED IN THE PLANS. THE SAW CUT SHALL BE MADE IN THE COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK, AND THE COMPLETED SAWCUT SHALL BE FILLED WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION TT-S-00227E. THE BOTTOM HALF INCH OF THE ONE INCH DEEP SAWED JOINT IN BOTH THE INSIDE AND OUTSIDE FACES OF THE PARAPET SHOULD BE LEFT UNSEALED TO ALLOW ANY WATER WHICH MAY ENTER THE JOINT TO ESCAPE. ALL LABOR, TOOLS, EQUIPMENT, MATERIAL AND INCIDENTALS NECESSARY TO COMPLETE THIS WORK ARE INCLUDED UNDER ITEM 842 CLASS S CONCRETE, SUPERSTRUCTURE.

ITEM 203 EMBANKMENT AS PER PLAN

ALL FILL MATERIAL FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT PLACED BETWEEN STATIONS 164+75 TO 169+25 AND FOR FILLING THE EXCAVATION VOID CREATED BY ITEM 503 SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 5/12 6/12 7/12 AND 8/12 SEE PROPOSAL NOTE.

ITEM 503, UNCLASSIFIED EXCAVATION ***, AS PER PLAN

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN 6 INCH LIFTS.

May 24, 1999 Q:\BRIDGE\Fay35\3888sgn.dwg

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

CHECKED VK | REVISED

DESIGN AGENCY
ENGINEERING ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

STRUCTURE FILE NO.

GENERAL NOTES
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57

3 / 12

457
522

ESTIMATED QUANTITIES

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	ABUT.	PIER	SUPER.	GEN.
503	21101	250	CU YD	UNCLASSIFIED EXCAVATION, AS PER PLAN	157	93		
505	11100	LUMP	LUMP	PILE DRIVING EQUIPMENT MOBILIZATION				LUMP
507	00500	1910	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES DRIVEN	1100	810		
507	00550	2155	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES FURNISHED	1210	945		
507	50500	35	EACH	STEEL PILE SPLICES	22	13		
SPECIAL	51267500	664	SQ YD	SEALING OF CONCRETE SURFACES *	71	98	495	
516	13600	134	SQ FT	1" PREFORMED EXPANSION JOINT FILLER	134			
516	13200	83	SQ FT	1/2" PREFORMED EXPANSION JOINT FILLER	83			
516	44101	4	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.		4		
				20" x 26" x 2" NEOPRENE WITH 2" x 21" x 27" STEEL				
				LOAD PLATE, AS PER PLAN *				
516	30501	114	LIN. FT.	PVC WATERSTOP, AS PER PLAN	114			
518	21200	88	CU YD	POROUS BACKFILL WITH FILTER FABRIC	88			
518	40000	137	LIN FT	6" PERFORATED CORRUGATED PLASTIC PIPE,	137			
518	40010	46	LIN FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE,	46			
				INCLUDING SPECIALS				
816	00600	LUMP	SUM	FIELD PAINTING OF NEW STEEL INTERMEDIATE AND FINISH COAT			LUMP	
				SYSTEM IZEU.				
844	48000	341	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (DECK)			341	
844	48020	68	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (PARAPET)			68	
844	49000		LUMP	HIGH PERFORMANCE CONCRETE, TRIAL MIX				
844	49010		LUMP	HIGH PERFORMANCE CONCRETE, TESTING				
842	41000	29	CU YD	CLASS C CONCRETE, PIER CAPS AND COLUMNS		29		
842	43500	110	CU YD	CLASS C CONCRETE, ABUTMENT INCLUDING FOOTING	110			
842	46500	47	CU YD	CLASS C CONCRETE, FOOTING		47		
863	10080	LUMP	SUM	STRUCTURAL STEEL MEMBER LEVEL FOUR (4) FABRICATION			LUMP	
863	20000	1764	EACH	WELDED STUD SHEAR CONNECTOR			1764	
				Computed by: S.P. Date: 6-8-94				
				Checked by: V.K. Date: 7-8-94				

* SEE PROPOSAL NOTE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
VK			

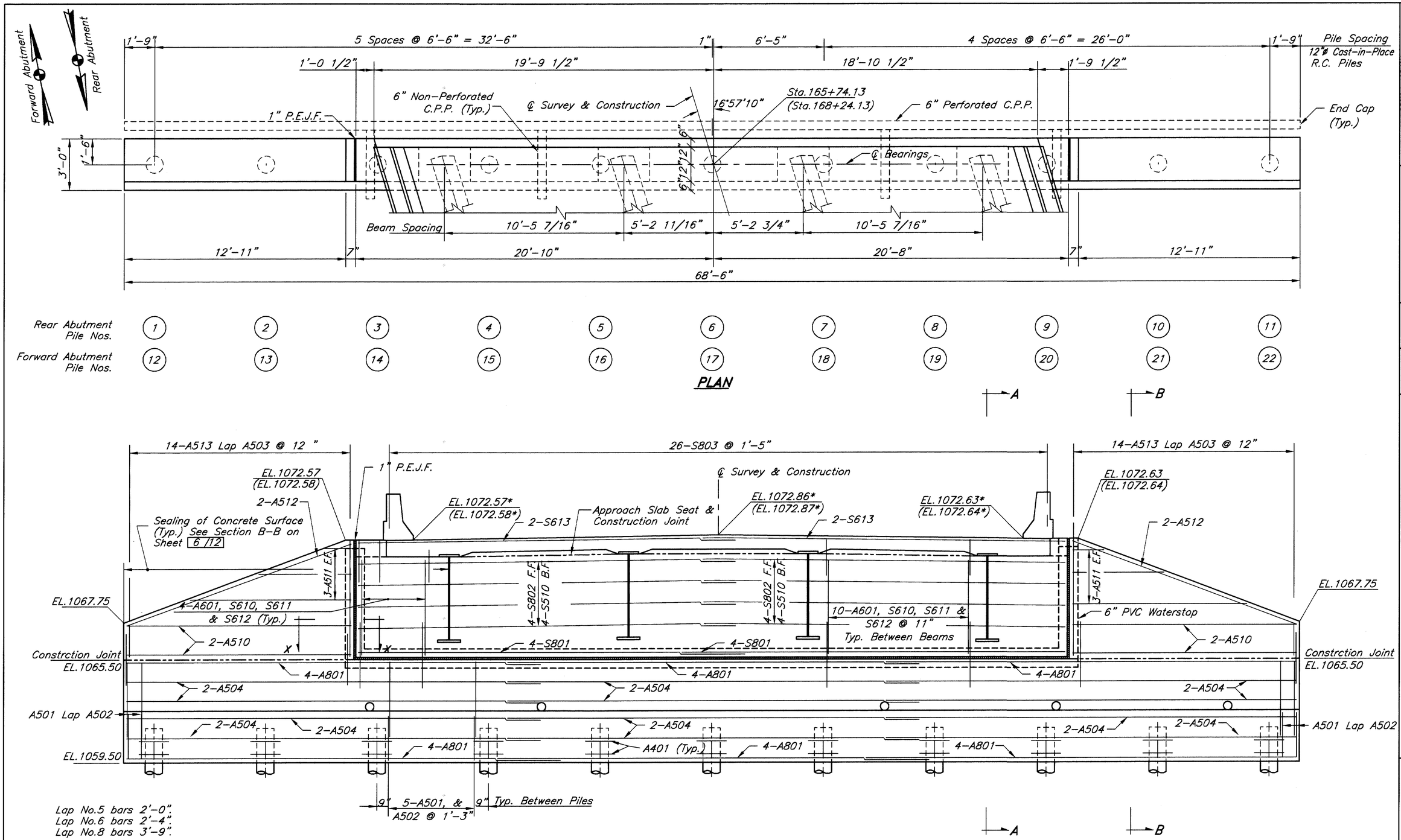
STRUCTURE FILE NO.

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57

C:\BRIDGE\Fay35\3886eqn.dwg
May 24, 1999

Revised 8/11/00 by E.R.P.



PLAN

Lap No.5 bars 2'-0".
Lap No.6 bars 2'-4".
Lap No.8 bars 3'-9".

ABBREVIATIONS

F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Plastic Pipe
P.E.J.F. = Preformed Expansion Joint Filler

CONCRETE FOR ABUTMENT WINGWALLS ABOVE BRIDGE SEAT CONSTRUCTION JOINT, SHALL NOT BE PLACED UNTIL THE REINFORCEMENT FOR DECK SLAB AND DIAPHRAGM HAS BEEN ERECTED.

ELEVATION

* Elevations are given @ ℄ Bearings

Elevations & Station in ( ) are for Forward Abutment only.

For notes, minimum lap lengths and Sections A-A, B-B & X-X see sheet 6 /12

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

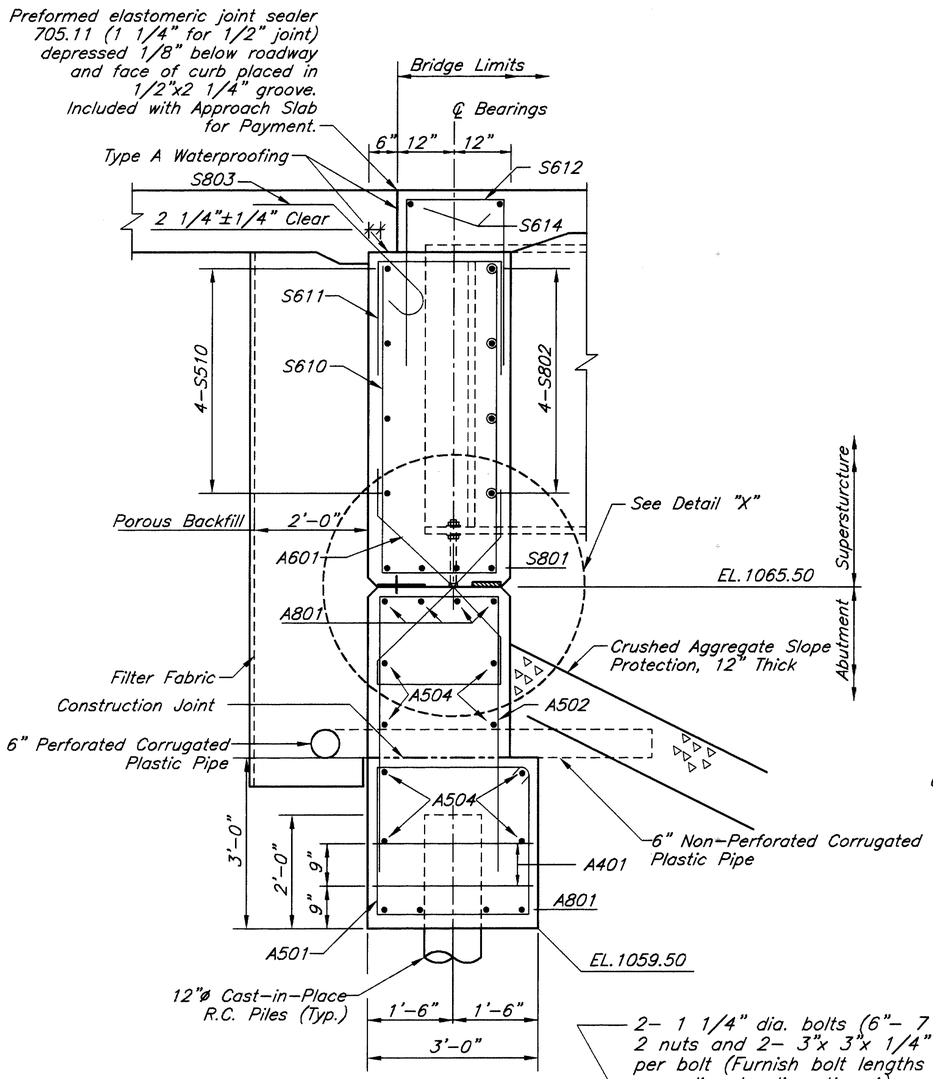
DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.

ABUTMENT DETAILS
Bridge No. FAY-35-0518
S.R.729 OVER US 35

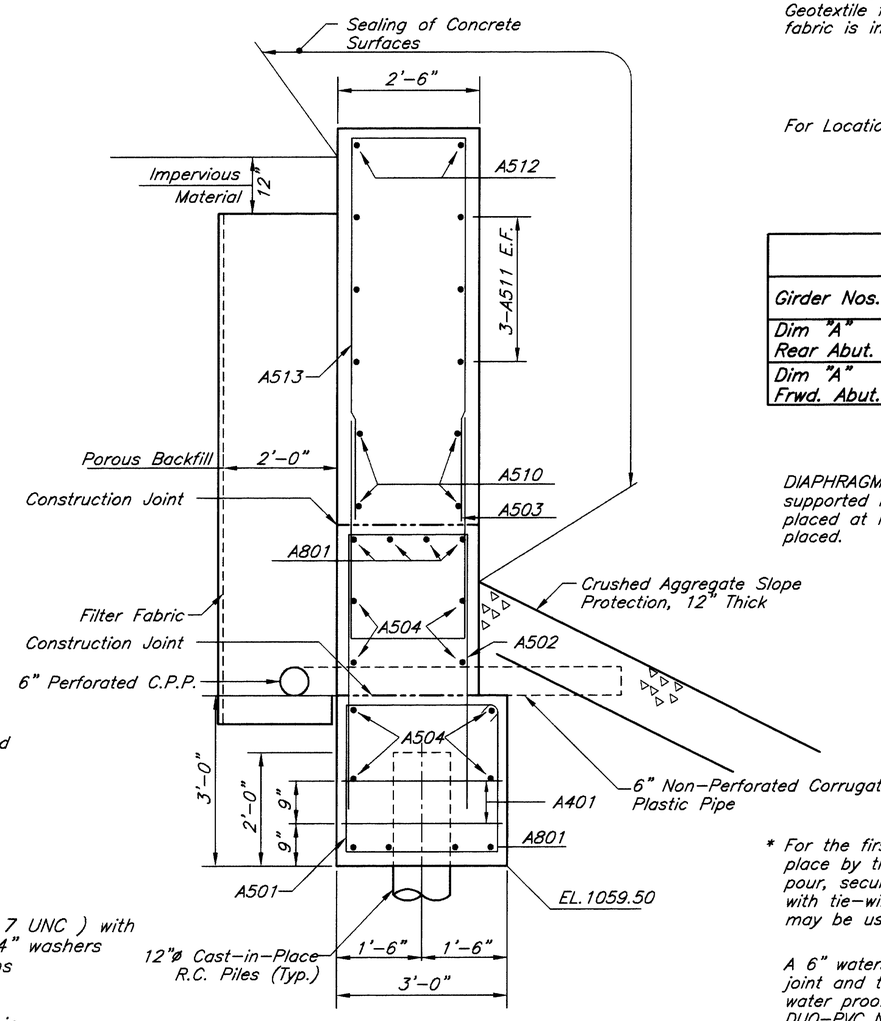
FAY-35-2.57

5 / 12

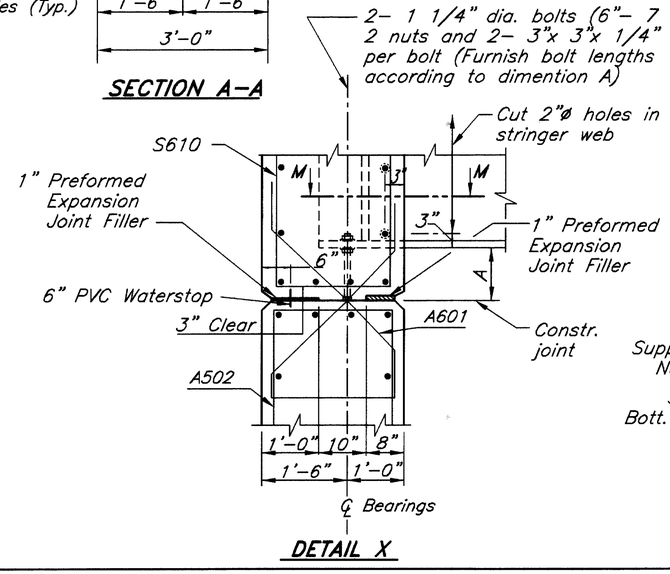
459
522



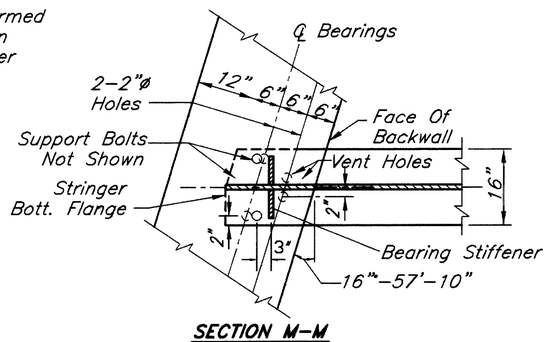
SECTION A-A



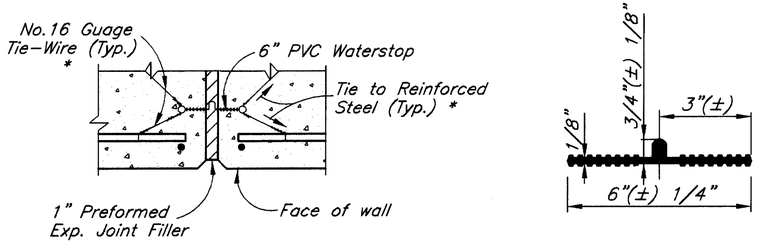
SECTION B-B



DETAIL X



SECTION M-M



WATERSTOP DETAIL

POROUS BACKFILL *with filter fabric shall extend upward to the plane of the subgrade,to one foot below the embankment surface, and laterally to the ends of the wingwalls. Geotextile fabric shall conform with 712.09, Type A. Geotextile fabric is included with porous backfill for payment.*

For Location of Sections A-A, B-B & X-X see sheet 5|12

Bearing Support-Dim. "A"				
Girder Nos.	1	2	3	4
Dim "A" Rear Abut.	1'-0 1/2"	1'-2 1/8"	1'-1 15/16"	11 7/8"
Dim "A" Frwd. Abut.	1'-0"	1'-2 1/16"	1'-2 1/4"	1'-0 9/16"

DIAPHRAGM concrete encasing the structural member sections supported in semi-integral and integral type abutments shall be placed at least 48 hours before the actual deck concrete is placed.

* *For the first pour, the waterstop should be held securely in place by the use of split forms and tie-wires. For the second pour, secure the free end of the waterstop in proper position with tie-wires. Alternate methods as approved by the Engineer, may be used to insure the correct positioning of the waterstop.*

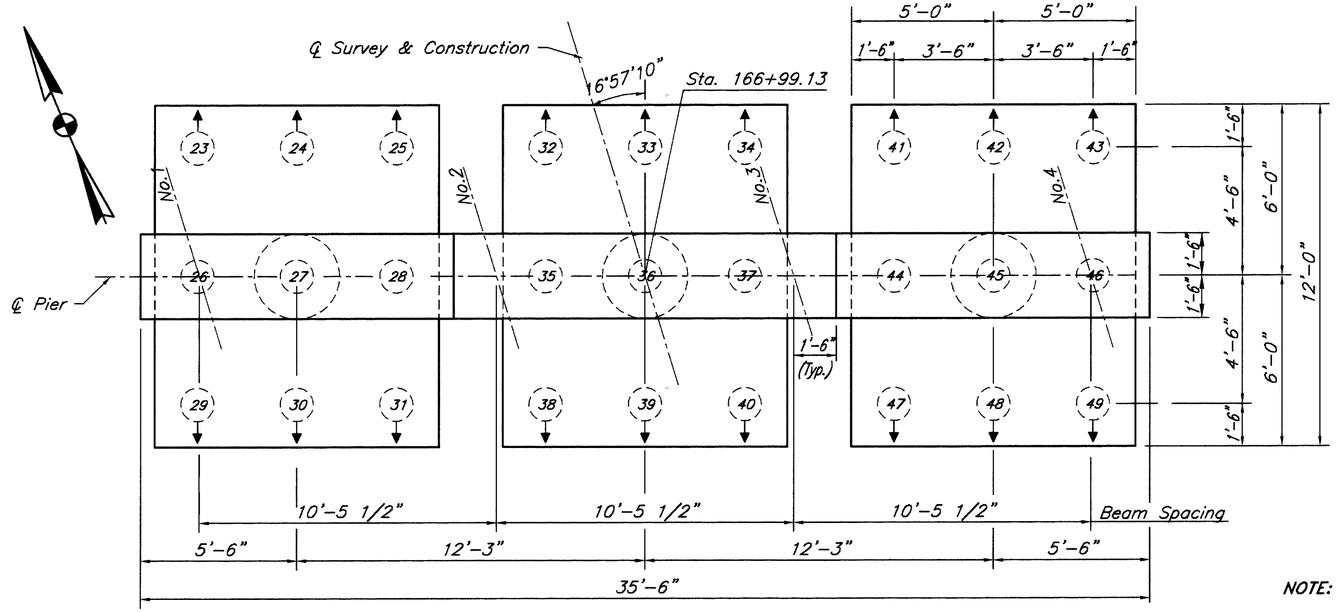
A 6" waterstop shall be centered on the bridge seat construction joint and the backwall expansion joint to form a continuous water proof seal. The waterstop shall be W.R. Meadows Sealtight DUO-PVC No. 6180-D or No. 6180-ND or an approved alternate. The waterstop shall be capable of accomodating 1 1/2" of movement.

DESIGN AGENCY: ERIKSSON ENGINEERING LTD. 1523 CHESAPEAKE AVENUE COLUMBUS, OH 43212
DESIGNED: RT | DRAWN: RT | CHECKED: SP/VK | REVIEWED: CEE | DATE: 2-24-99 | REVISED:
STRUCTURE FILE NO.

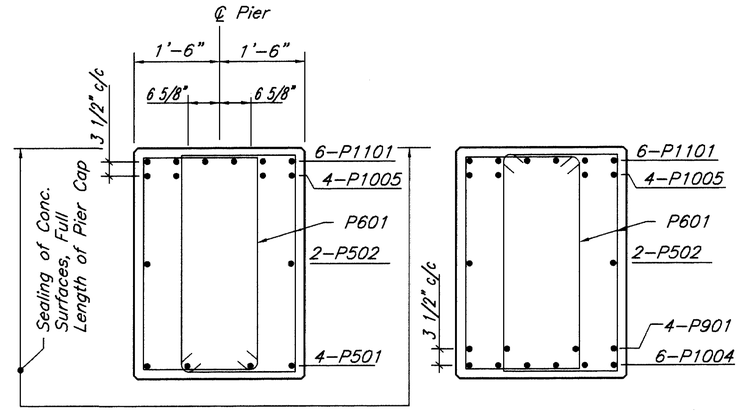
ABUTMENT DETAILS

Bridge No. FAY-35-0518
C.R.729 OVER U.S. 35

FAY-35-2.57



PLAN

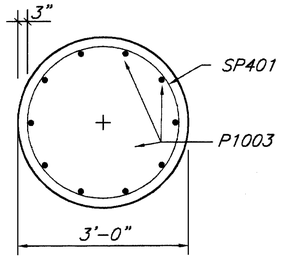


SECTION A–A

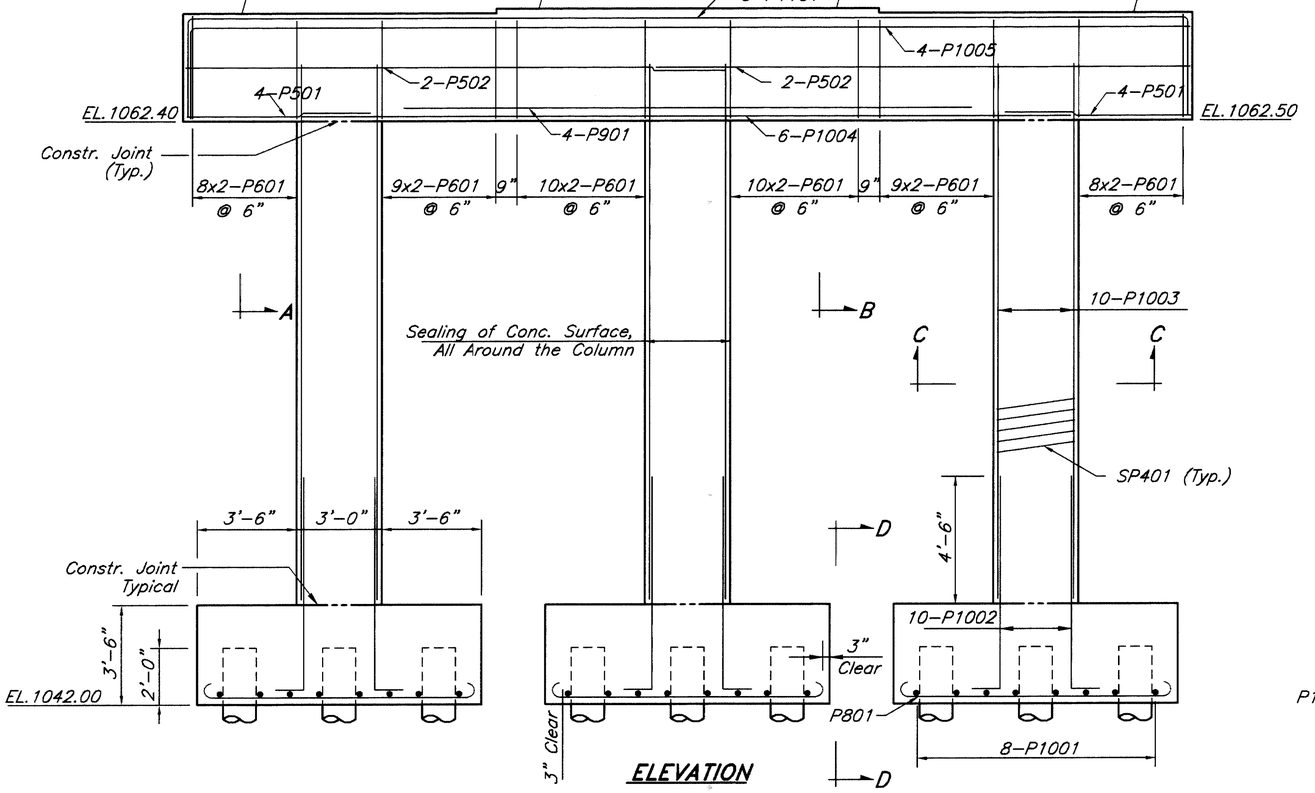
SECTION B–B

NOTE: Footing & Pile Dimensions are Typical for All Footings.

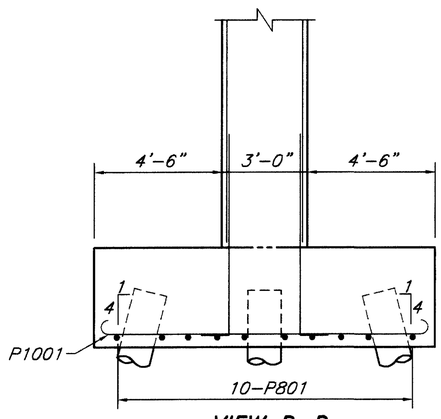
Indicates Piles Battered at 1:4



SECTION C–C



ELEVATION



VIEW D–D

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

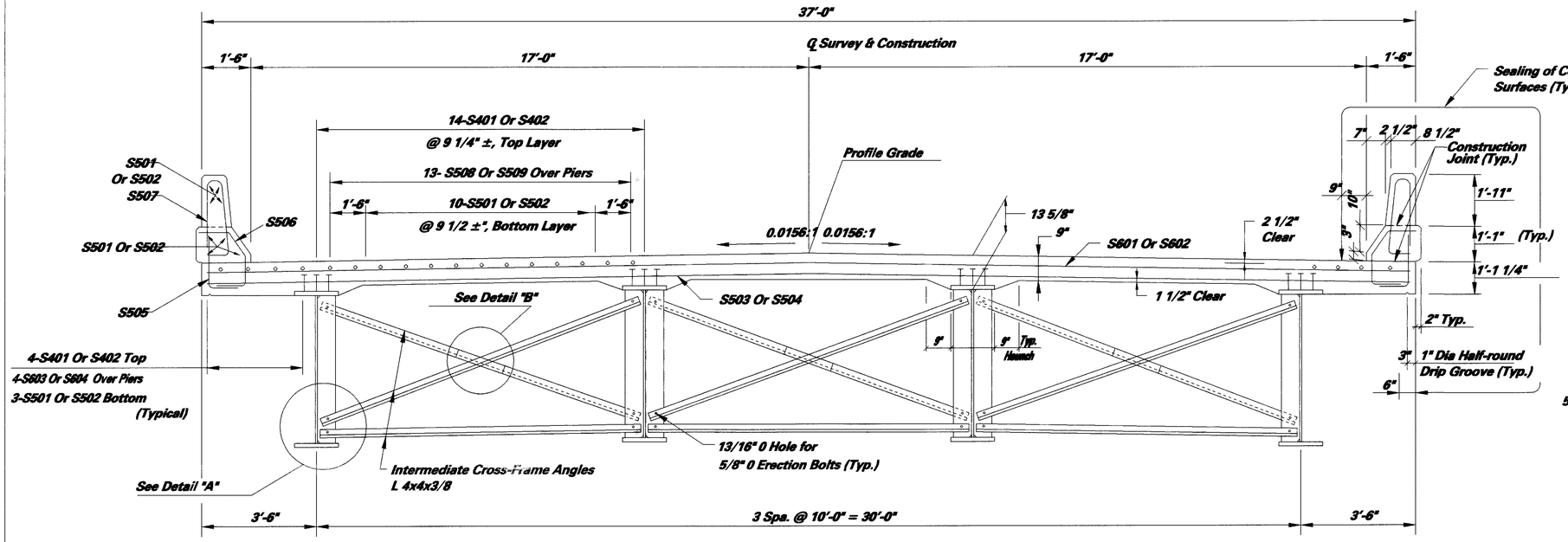
DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVIEWED CEE
DATE 2-24-99
REVISED
STRUCTURE FILE NO.3000214

PIER DETAILS
Bridge No. FAY-35-0518
S.R.729 OVER US 35

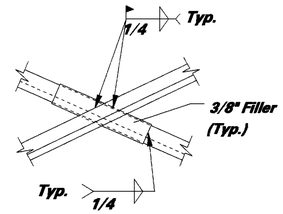
FAY-35-0518

7 / 12

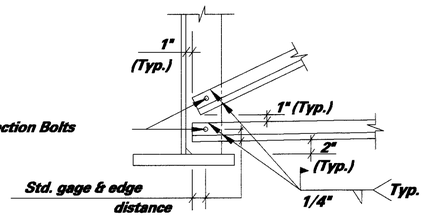
461
522



TRANSVERSE SECTION



DETAIL B



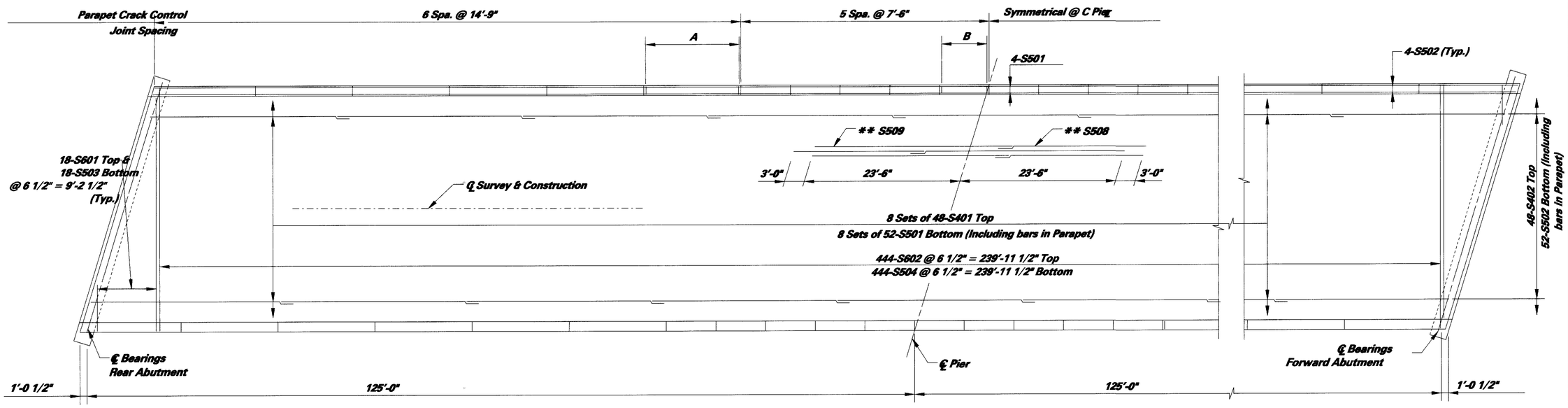
DETAIL A

A = 11-S505, S506 & S507 @ 1'-5"

B = 6-S505, S506 & S507 @ 1'-5"

Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

For Notes See Sheet 9/12



DECK REINFORCING PLAN

** 47-S508 and 47-S509 required over pier

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DATE 2-24-09
REVIEWED CEE
DRAWN RT REVISED
DESIGNED RT CHECKED SP/VK

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0518
SR 729 OVER US 35

FAY-35-2.57

8 | 12

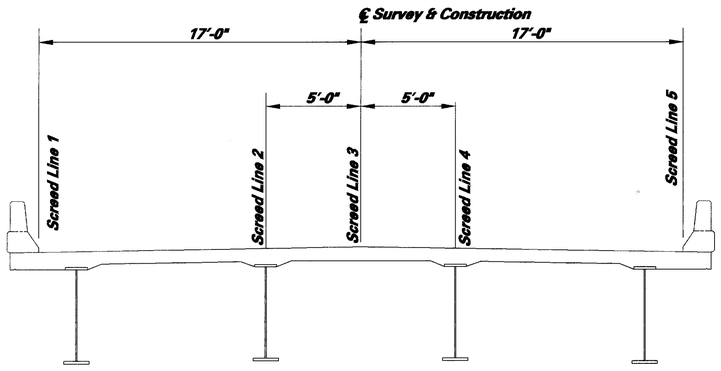
462
522

SCREED ELEVATION TABLE

		SCREED LINE 1		SCREED LINE 2		SCREED LINE 3		SCREED LINE 4		SCREED LINE 5	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	165+79.31	1072.63	165+75.65	1072.79	165+74.13	1072.86	165+72.61	1072.78	165+68.95	1072.57
	0.20 L	166+04.31	1072.83	166+00.65	1073.01	165+99.13	1073.08	165+97.61	1072.99	165+93.95	1072.79
	0.40 L	166+29.31	1072.96	166+25.65	1073.14	166+24.13	1073.21	166+22.61	1073.13	166+18.95	1072.92
	0.60 L	166+54.31	1072.99	166+50.65	1073.18	166+49.13	1073.25	166+47.61	1073.17	166+43.95	1072.97
	0.80 L	166+79.31	1072.97	166+75.65	1073.16	166+74.13	1073.23	166+72.61	1073.15	166+68.95	1072.96
	0.00 L	167+04.31	1072.95	167+00.65	1073.14	166+99.13	1073.21	166+97.61	1073.14	166+93.95	1072.95
Span No.2	0.20 L	167+29.31	1072.96	167+25.65	1073.16	167+24.13	1073.24	167+22.61	1073.16	167+18.95	1072.97
	0.40 L	167+54.31	1072.98	167+50.65	1073.17	167+49.13	1073.25	167+47.61	1073.18	167+43.95	1073.00
	0.60 L	167+79.31	1072.93	167+75.65	1073.13	167+74.13	1073.22	167+72.61	1073.14	167+68.95	1072.96
	0.80 L	168+04.31	1072.79	168+00.65	1073.00	167+99.13	1073.08	167+97.61	1073.01	167+93.95	1072.84
	1.00 L	168+29.31	1072.58	168+25.65	1072.79	168+24.13	1072.87	168+22.61	1072.90	168+18.95	1072.64

SCREED ELEVATIONS are for the concrete surface before the concrete is placed. Allowance has been made for the anticipated deflection due to deck concrete.

L = Span Length



SCREED LINE LOCATIONS

NOTES:

THE DISTANCE SHOWN FROM THE TOP OF THE DECK SLAB TO THE BOTTOM OF THE TOP FLANGE IS THE THEORETICAL DESIGN DIMENSION. THE QUANTITY OF DECK CONCRETE TO BE PAID FOR SHALL BE BASED UPON THIS DIMENSION, EVEN THOUGH DEVIATION FROM IT MAY BE NECESSARY BECAUSE THE TOP FLANGE OF THE GIRDER MAY NOT HAVE THE EXACT CAMBER OR CONFORMATION REQUIRED TO PLACE IT PARALLEL TO THE FINISHED GRADE. DEDUCTION SHALL BE MADE FOR VOLUME OF ENCASED STEEL PLATES AS PER 842.

CONCRETE DECK HAUNCH WIDTH

A HAUNCH WIDTH OF 9 INCHES SHALL BE USED FOR COMPUTING QUANTITY OF CONCRETE. HOWEVER, THE HAUNCH WIDTH MAY VARY BETWEEN 6 AND 12 INCHES.

ERECTION BOLTS:
THE HOLE DIAMETER IN THE CROSS FRAMES AND GIRDER STIFFENERS SHALL BE 3/16" LARGER THAN THE DIAMETER OF THE ERECTION BOLTS. UNLESS REPLACED BY PERMANENT HIGH STRENGTH BOLTS, ERECTION BOLTS SHALL REMAIN IN PLACE. LOCK WASHERS SHALL BE FURNISHED FOR OTHER THAN FULLY TORQUED HIGH STRENGTH ERECTION BOLTS. BOLTS SHALL BE FURNISHED AS PART OF ITEM 863.

IN LIEU OF ERECTION BOLTS AND AT THE OPTION OF THE CONTRACTOR, ALTERNATIVE MEANS OF TEMPORARY BRACING MAY BE USED SUBJECT TO THE APPROVAL OF THE DIRECTOR (501.06).

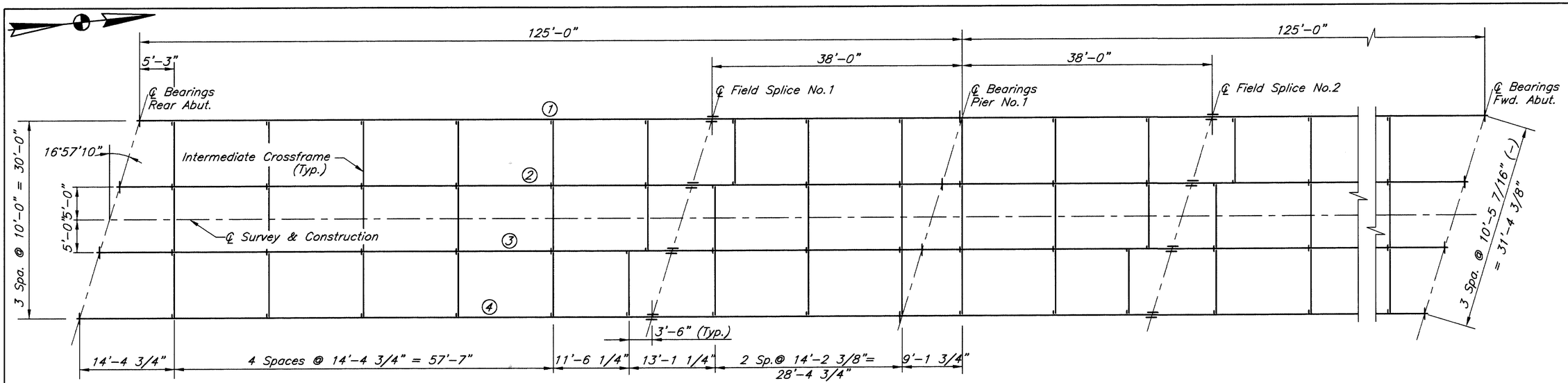
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

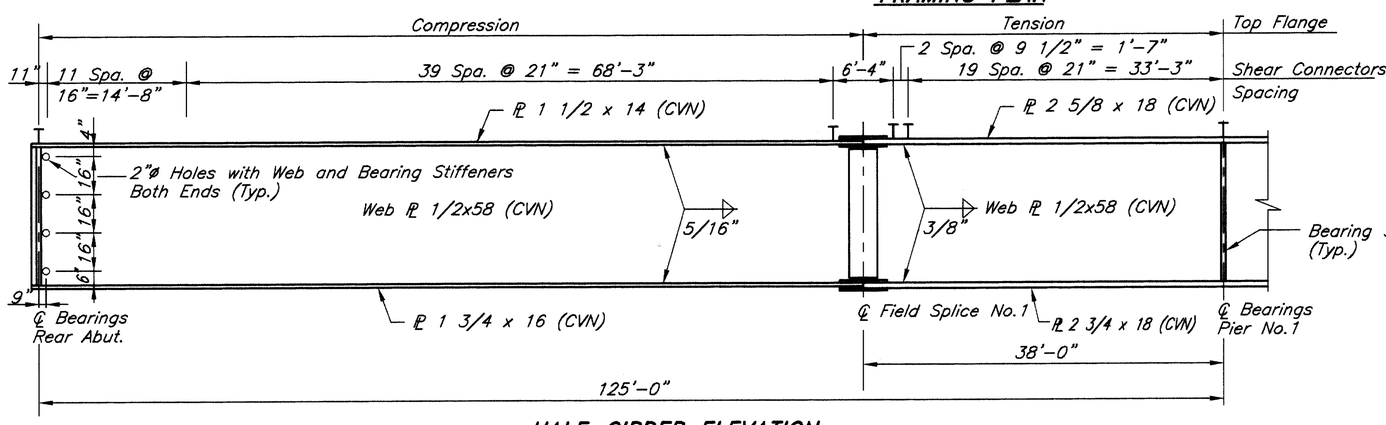
STRUCTURE FILE NO.

SCREED ELEVATIONS
Bridge No. FAY-35-0518
S.R.729 Over US 35

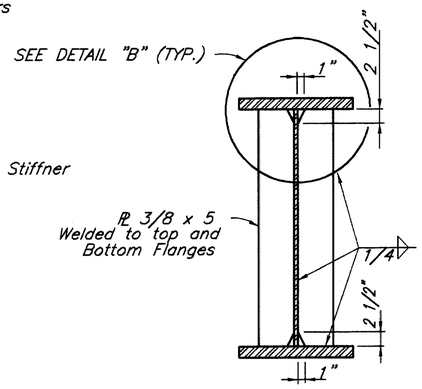
FAY-35-2.57



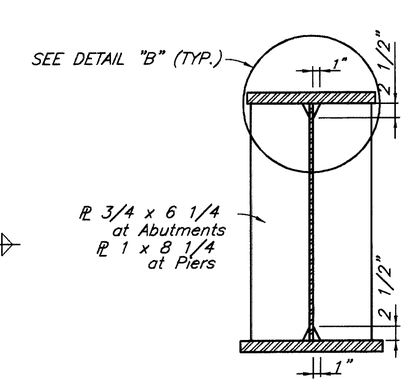
FRAMING PLAN



HALF-GIRDER ELEVATION



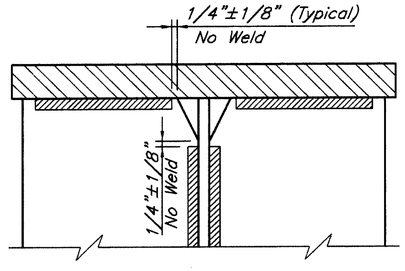
CROSSFRAME CONNECTION PLATE DETAIL



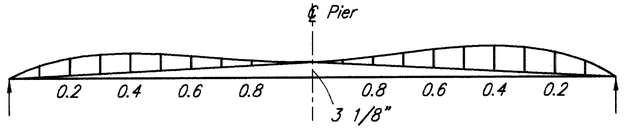
BEARING STIFFENER DETAIL

DEFLECTION AND CAMBER

	Points	Span No.1								
		.10	.20	.30	.40	.50	.60	.70	.80	.90
INTERIOR BEAMS	Deflection due to weight of steel	3/16	5/16	3/8	7/16	3/8	5/16	1/4	1/8	1/16
	Deflection due to remaining dead load	5/8	1 3/16	1 1/2	1 5/8	1 9/16	1 1/4	7/8	7/16	1/8
	Adjustment for Vertical curve	3/8	5/8	7/8	1	1 1/16	1	7/8	5/8	3/8
	Net Total Camber	1 3/16"	2 1/8"	2 3/4"	3 1/16"	3"	2 9/16"	2"	1 3/16"	9/16"



DETAIL "B"



NOTES :

WELDED ATTACHMENT of supports for concrete deck finishing machine may be made to areas of the flanges designated "Compression". Attachments shall not be made to areas designated "Tension". Fillet welds to compression flanges shall be not closer than 1" from edge of the flange, be not more than 2" long and be not smaller than the minimum size required by AASHTO.

CHARPY V-NOTCH TOUGHNESS REQUIREMENT

All girder components, i.e., flange plates, web plates and splice plates shall meet minimum notch toughness requirements as specified in 711.01 of CMS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		STRUCTURE FILE NO.
SP/VK			

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57

10 / 12

464
522

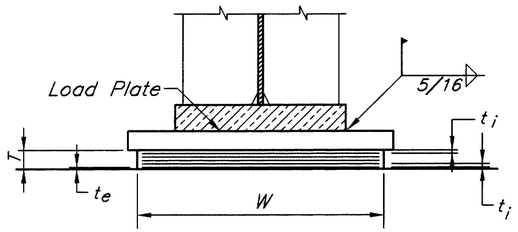
May 24, 1999 Q:\BRIDGE\Fay35\3595efr.dwg

LAMINATED ELASTOMERIC BEARINGS

LOCATION	DIMENSIONS (Inch)					N	STEEL LOAD PLATE	REACTIONS		MAXIMUM DESIGN LOAD
	L	W	t	t	T			DL	LL	
Pier Expansion Brngs.	20	26	0.285	0.203	2	5	21" x 27" x 2"	375 k	117 k	492 k

t = Thickness of Internal Layer
t = Thickness of External Layer
T = Total Thickness of Elastomeric Bearing

N = No. of Steel Laminates
Internal Steel Laminate Thickness = 0.0747"
Durometer of Elastomer = 50 Durometer



LAMINATED ELASTOMERIC EXPANSION BEARING

LOAD PLATE

The steel load plate shall be the same material as the attached structural steel and be similarly cleaned and coated. Surface preparation and priming shall be done in the shop and be included in the price bid for the bearings. Field coats shall be included in the price bid for painting main structural steel.

The steel load plate shall be bonded by vulcanization to the elastomer during the molding process.

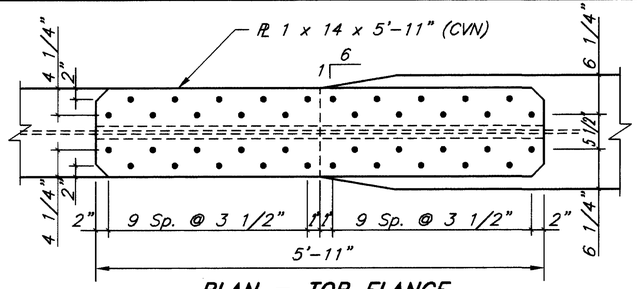
Welding of the load plate to the superstructure shall be controlled so that the plate temperature at the elastomer bonded surface shall not exceed 300° F as determined by the use of pyrometric sticks or other temperature monitoring devices.

BASIS OF PAYMENT

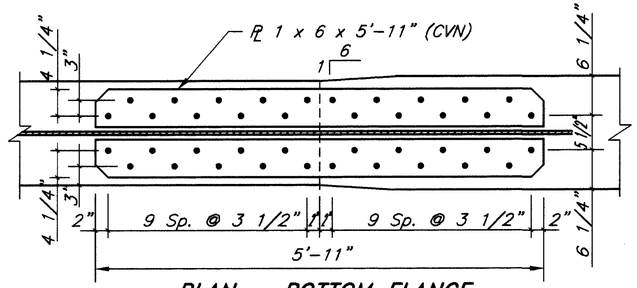
The unit bid price shall include all materials, labor and incidentals necessary to furnish and install laminated elastomeric bearings either fixed or expansion. Payment will be made at the contract price for Item 516, each, Laminated Elastomeric Bearings.

ELASTOMERIC BEARINGS

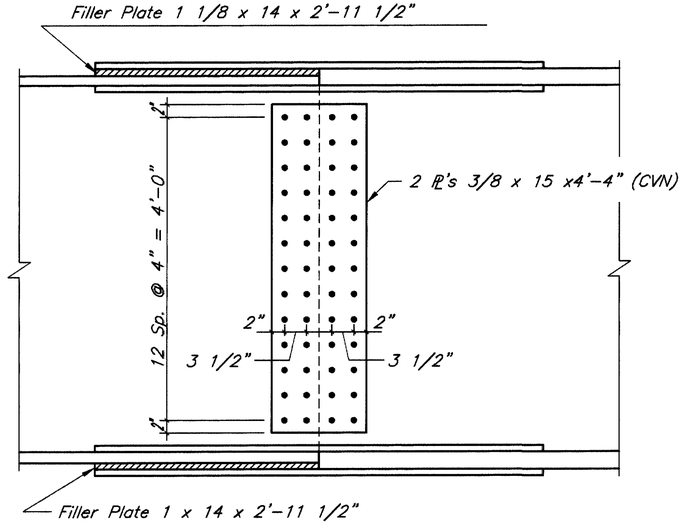
Elastomeric bearings shall comply with Item 516 and articles 18.2.5 through 18.2.8, Section 18, Bearing Devices, Division II, Construction, of the AASHTO Specifications for Highway Bridges. Bearings shall be Grade 3, 50-durometer elastomer and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the price bid for the bearings.



PLAN – TOP FLANGE



PLAN – BOTTOM FLANGE



TYPICAL FIELD SPLICE DETAILS

HIGH STRENGTH BOLTS

High strength bolts shall be 1" diameter A325 unless otherwise noted.

* CHARPY V-NOTCH TOUGHNESS REQUIREMENT

Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in 711.01 of CMS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | CHECKED SP/VK | REVIEWED CEE | DATE 2-24-99 | REVISED | STRUCTURE FILE NO.

BEARING & SPLICE DETAILS
Bridge No. FAY-35-0518
SR 729 Over US 35

FAY-35-2.57

11 / 12

465
522

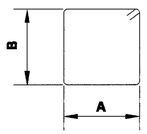
REINFORCING STEEL LIST

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				ABUTMENTS					
A401	44	9-0	265	1	1-9	2-6			
A501	108	11-1	1248	1	2-8	2-7			
A502	108	11-7	1305	2	2-2	4-10			
A503	56	9-11	579	2	2-2	4-0			
A504	48	24-1	1206	STR					
A510	16	13-0	217	STR					
A511	24	2-0 to	144	STR					8 Sets of 3
		9-6							Vary "B" by 3'-9"
A512	8	13-9	115	STR					
A513	56	5-11 to	628	2	2-2	2-0 to			4 Sets of 14
		15-7				6-10			Vary "B" by 4 1/2"
									(†)
A601	76	10-9	1227	4					
A801	48	25-3	3236	STR					
			10170	LBS					
				PIER					
P501	8	6-6	54	STR					
P502	4	18-7	78	STR					
P601	108	12-0	1947	1	2-0 1/2	3-8			
P801	30	11-4	908	10	9-6				
P901	4	20-0	272	STR					
P1001	24	14-4	1480	10	11-6				
P1002	30	9-3	1194	5	1-10	7-9			
P1003	30	20-0	2582	STR					
P1004	6	27-6	710	STR					
P1005	4	40-0	688	2	34-6	3-1			
P1101	6	41-6	1323	2	35-2	3-6			
Spirals			967						
			12203	LBS					

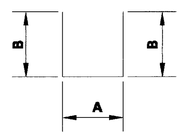
Dimensions shown are out to out.

MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				SUPERSTRUCTURE (AT ABUTMENT)					
S510	16	21-7	360	STR					
S610	76	12-0	1370	2	2-2	5-1			
S611	76	7-6	856	2	2-2	2-10			
S612	76	7-6	856	2	1-8	3-1			
S613	8	21-9	261	STR					
S801	16	22-6	961	STR					
S802	16	22-6	961	STR					
S803	52	5-0	694	3	2-8	1-5			
			6319	LBS					
				SUPERSTRUCTURE (DECK)					
S401	384	30-0	7695	STR					
S402	48	25-1	804	STR					
S501	416	30-0	13017	STR					
S502	52	27-9	1505	STR					
S503	36	5-7 to	776	STR					2 SETS OF 18
		35-9							VARY BY 1'-9" ( ) †
S504	444	36-8	16980	STR					
S505	384	2-6	1001	5	10 1/2	1-9			
S506	384	3-4	1335	6	1-0				
S507	384	6-0	2403	7					
S508	47	22-0	1078	STR					
S509	47	30-0	1471	STR					
S601	36	5-7 to	1117	STR					2 SETS OF 18
		35-9							VARY BY 1'-9" ( ) †
S602	444	36-8	24468	STR					
			73635	LBS					

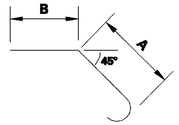
ALL BARS SHALL BE EPOXY COATED



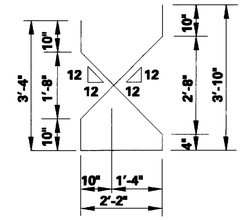
TYPE 1



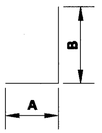
TYPE 2



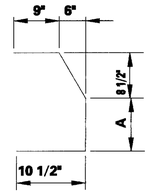
TYPE 3



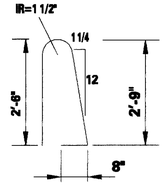
TYPE 4



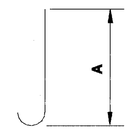
TYPE 5



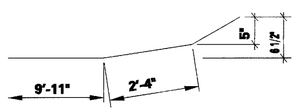
TYPE 6



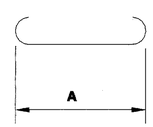
TYPE 7



TYPE 8



TYPE 9



TYPE 10

MARK	NO.	LENGTH	WEIGHT	SHAPE	REMARKS
SP401	3	17-1	967	BT.	
			967	LBS.	

SP401 - Core Diameter 30" c/c

Pitch 4 1/2"

Other Details in Accordance with C.R.S.I. Standard Practice.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Four steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 3.20 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED CEE | DATE 2-24-99 | STRUCTURE FILE NO.
DRAWN SP | REVISED
DESIGNED SP | CHECKED VK

FAYETTE COUNTY

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0518
S.R.729 OVER U.S.R.35

FAY-35-2.57

12 / 12

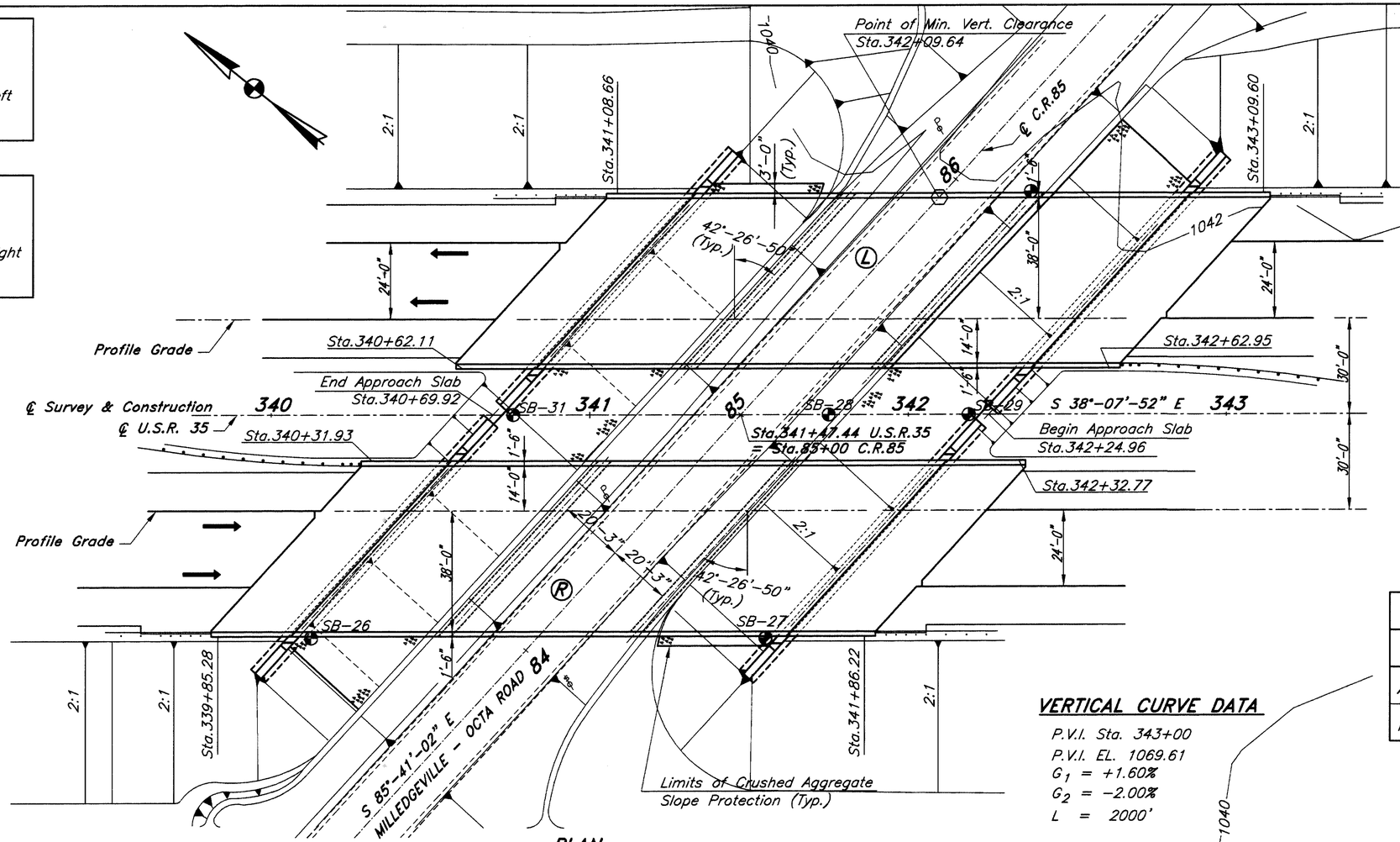
466
522

B.M.–TR102

Iron Pin With Cap
Sta.72+35.45, 15.7' Left
Elev. = 1040.405

B.M.–TR101

Iron Pin With Cap
Sta. 88+95.70, 7.5' Right
Elev. = 1041.523



PLAN

EARTHWORK LIMITS shown are approximate. Actual slopes shall conform to plan cross–sections.

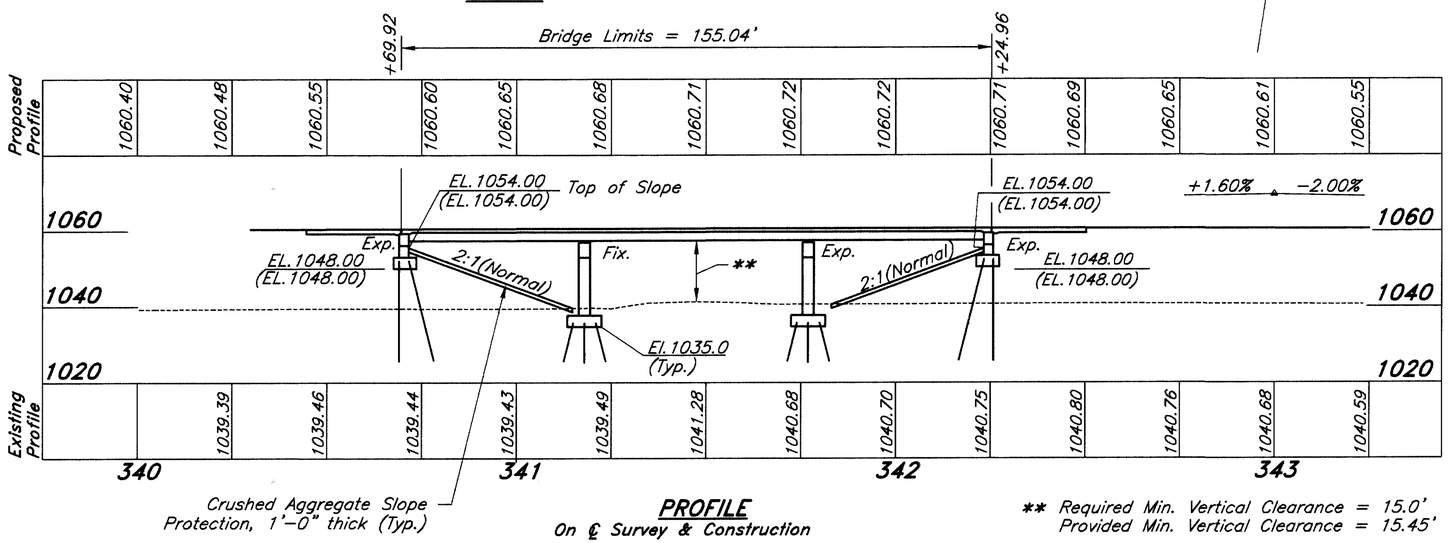
SB, Indicates Boring Location

DESIGN TRAFFIC

1997 ADT 7520
2017 ADT 9910
18% Trucks

VERTICAL CURVE DATA

P.V.I. Sta. 343+00
P.V.I. EL. 1069.61
G_1 = +1.60%
G_2 = –2.00%
L = 2000'



PROFILE
On ℄ Survey & Construction

** Required Min. Vertical Clearance = 15.0'
Provided Min. Vertical Clearance = 15.45'

Elevations in ( ) are for Right Bridge.

PILE DATA

Location	Type	Estimated Pile Length
Abutments	12"ø Conc.	35'
Piers	12"ø Conc.	25'

PROPOSED STRUCTURE

TYPE : Continuous Steel Beams, (A572 Painted) Reinforced Conc. Substructures
SPAN: 47'–0", 59'–0" & 47'–0" c/c Bearings
ROADWAY: 52'–0" Toe/Toe Parapets
LOADING: HS20–44 (Case II) and the Alternate Military Loading
SKEW: 42°–26'–50" L.F. Both Bridges
WEARING SURFACE: Monolithic Concrete
APPROACH SLABS: 25' Long (AS–1–81)
ALIGNMENT: Tangent
CROWN : 0.0156:1
LATITUDE : 39°–35'–42.71"N
LONGITUDE : 83°–36'–01.55"W
USGS QUADRANGLE : MILLEDGEVILLE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

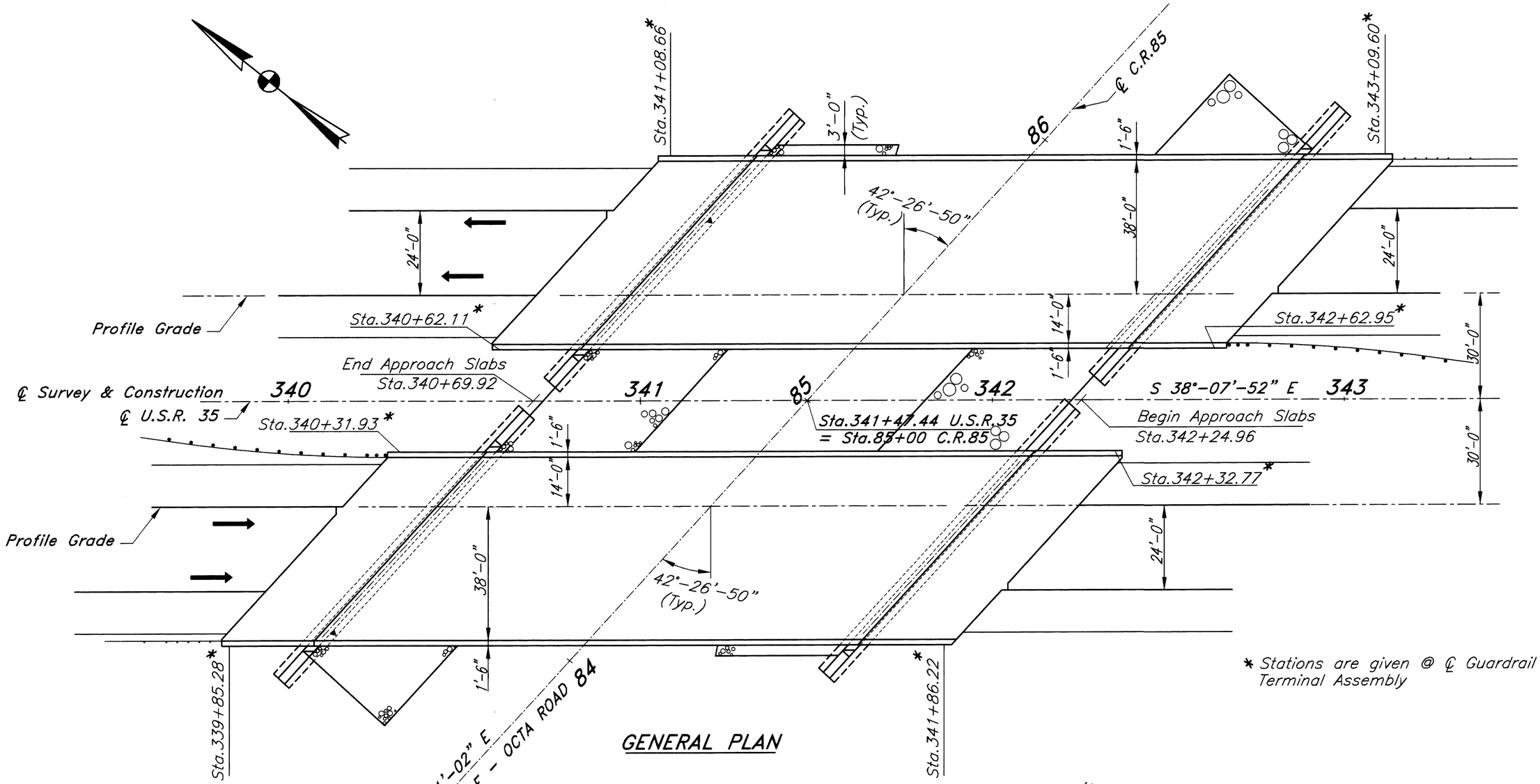
DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2–24–99
CHECKED	REVISED		
VK			

STRUCTURE FILE NO.2400448

FAYETTE COUNTY
STA.340+69.92
STA.342+24.96

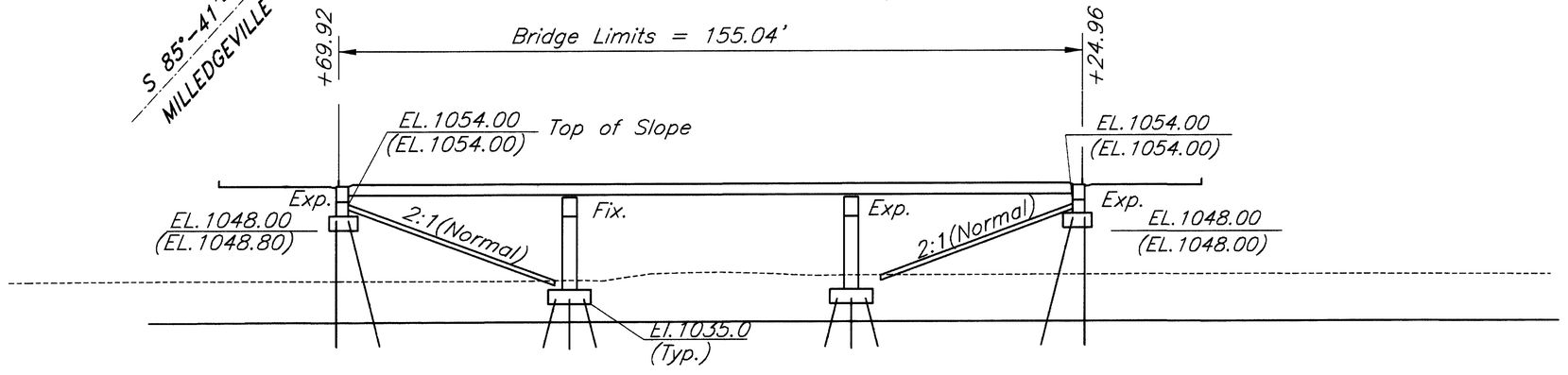
SITE PLAN
BRIDGE NO. FAY–35–0644 L. & R.
U.S.R.35 OVER MILLEDGEVILLE–OCTA ROAD

FAY–35–2.57



GENERAL PLAN

* Stations are given @ ℄ Guardrail Terminal Assembly



ELEVATION

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED	STRUCTURE FILE NO.2400448	
VK			

FAYETTE COUNTY
STA.340+69.92
STA.342+24.96

GENERAL PLAN
BRIDGE NO. FAY-35-0644 L & R.
U.S.R.35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

2 / 16

468
522

GENERAL NOTES

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 1992, INCLUDING THE 1993 INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING.
60 P.S.F. FUTURE WEARING SURFACE.

DESIGN STRESSES

HPC SS 844	-	COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS C	-	UNIT STRESS 1333 PSI. (SUBSTRUCTURE)
REINFORCING STEEL	-	ASTM A615, A616 OR A617 GRADE 60 UNIT STRESS 24,000 PSI
STRUCTURAL STEEL	-	ASTM A588-YIELD STRENGTH 50,000 P.S.I.

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81	DATED	9-15-94	SHEET NO. 1. AND 2.
BR-1	DATED	1-6-99	
GSD-1-96	DATED	2-12-97	SHEET NO. 1 & NO. 3

AND TO SUPPLEMENTAL SPECIFICATIONS :

816	DATED	4-21-97
842	DATED	1-6-99
844	DATED	1-6-99
863	DATED	10-12-99

DECK PROTECTION METHOD EPOXY COATED REINFORCING STEEL, TOP & BOTTOM MATS 2 1/2" CONCRETE COVER TOP, 1 1/2" COVER BOTTOM MATS AND SEALING OF CONCRETE SURFACES.

MONOLITHIC WEARING SURFACE IS ASSUMED FOR DESIGN PURPOSES TO BE 1".

PILE DRIVING CONSTRAINTS:

PRIOR TO DRIVING PILES, THE SPILL THROUGH SLOPES AND THE BRIDGE APPROACH EMBANKMENT BEHIND THE ABUTMENTS SHALL BE CONSTRUCTED UP TO THE LEVEL OF THE SUBGRADE ELEVATION FOR A MINIMUM DISTANCE OF 200 FEET BEHIND EACH ABUTMENT. THE EXCAVATION FOR THE ABUTMENT FOOTINGS AND THE INSTALLATION OF THE ABUTMENT PILES SHALL NOT BEGIN UNTIL AFTER THE ABOVE REQUIRED EMBANKMENT HAS BEEN CONSTRUCTED AND HAS UNDERGONE A WAITING PERIOD OF 30 DAYS.

PILE DESIGN LOADS (ULTIMATE BEARING VALUE)

THE ULTIMATE BEARING VALUE IS 54 TONS PER PILE FOR THE 12" CIP ABUTMENT PILES. THE ULTIMATE BEARING VALUE IS 90 TONS PER PILE FOR THE 12" CIP PIER PILES.

ABUTMENT PILES:
- 88 PILES 35 FEET LONG, ESTIMATED LENGTH
- 88 PILES OF ORDER LENGTH 40 FEET LONG
- 44 SPLICES

PIER PILES:
- 64 PILES 25 FEET LONG, ESTIMATED LENGTH
- 64 PILES OF ORDER LENGTH 30 FEET LONG
- 32 SPLICES

ABUTMENT BACKFILL

ABUTMENT BACKFILL ABOVE BEAM SEAT LEVEL SHALL BE PLACED SIMULTANEOUSLY AT BOTH ABUTMENTS TO EQUALIZE THE EARTH PRESSURE ON THE SUPERSTRUCTURE DURING CONSTRUCTION.

REINFORCING BAR SPLICES

REINFORCING BAR SPLICE LENGTHS SHALL CONFORM TO THE MINIMUM LENGTHS SPECIFIED BY 509.08 OF THE C.M.S. UNLESS OTHERWISE NOTED ON THE PLANS.

CONCRETE PARAPETS:

WITHIN 48 HOURS AFTER PLACEMENT OF PARAPET CONCRETE SAWCUT 1 INCH DEEP JOINTS INTO THE CONCRETE PARAPET AT LOCATIONS AS DETAILED IN THE PLANS. THE SAW CUT SHALL BE MADE IN THE COMPLETE CIRCUMFERENCE OF THE PARAPET, STARTING AND ENDING AT THE ELEVATION OF THE CONCRETE DECK, AND THE COMPLETED SAWCUT SHALL BE FILLED WITH A CAULKING MATERIAL CONFORMING TO FEDERAL SPECIFICATION TT-S-00227E. THE BOTTOM HALF INCH OF THE ONE INCH DEEP SAWED JOINT IN BOTH THE INSIDE AND OUTSIDE FACES OF THE PARAPET SHOULD BE LEFT UNSEALED TO ALLOW ANY WATER WHICH MAY ENTER THE JOINT TO ESCAPE.

ITEM 203 EMBANKMENT AS PER PLAN

ALL FILL MATERIAL FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT PLACED BETWEEN STATIONS 339 TO 344 AND FOR FILLING THE EXCAVATION VOID CREATED BY ITEM 503, SHALL BE 203 MATERIAL PLACED IN LIFTS NOT TO EXCEED A THICKNESS OF SIX (6) INCHES.

ITEM SPECIAL, SEALING OF CONCRETE SURFACES

A CONCRETE SEALER SHALL BE APPLIED TO THE SURFACES SHOWN ON SHEETS 7 16 & 16, AND 10 16. /
SEE PROPOSAL NOTE.

ITEM 842 CLASS C CONCRETE, ABUTMENT NOT INCLUDING FOOTINGS, AS PER PLAN

INSTALL A 3 FOOT WIDE STRIP, 3/32 INCH THICK, GENERAL PURPOSE HEAVY DUTY NEOPRENE SHEET WITH NYLON FABRIC REINFORCEMENT AT LOCATIONS SHOWN IN THE PLANS. SECURE THE 3 FOOT WIDE NEOPRENE SHEETING TO THE CONCRETE WITH 1 1/4" X 3/32" X 1/4" (LENGTH X SHANK DIAMETER X HEAD DIAMETER) #10 GALVANIZED SCREWS THROUGH A 1 INCH OUTSIDE DIAMETER, #10 GAGE GALVANIZED WASHER. MAXIMUM FASTENER SPACING IS 9 INCHES. OTHER SIMILAR GALVANIZED DEVICES WHICH WILL NOT DAMAGE EITHER THE NEOPRENE OR THE CONCRETE MAY BE USED SUBJECT TO THE APPROVAL OF THE ENGINEER.

CENTER THE NEOPRENE STRIPS ON ALL JOINTS. FOR HORIZONTAL JOINTS, SECURE THE HORIZONTAL NEOPRENE STRIP BY USING A SINGLE LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE TOP OF THE NEOPRENE STRIP. FOR THE VERTICAL JOINTS SECURE THE VERTICAL NEOPRENE STRIP BY USING A SINGLE VERTICAL LINE OF FASTENERS, STARTING AT 6 INCHES (+/-) FROM THE VERTICAL EDGE OF THE NEOPRENE STRIP NEAREST TO THE CENTERLINE OF ROADWAY. FOR VERTICAL JOINTS, INSTALL 2 ADDITIONAL FASTENERS AT 6 INCHES CENTER TO CENTER ACROSS THE TOP HALF OF THE NEOPRENE STRIP ON THE SIDE OF TH NEOPRENE STRIP AS THE SINGLE VERTICAL ROW OF FASTENERS.

THE VERTICAL NEOPRENE STRIPS SHOULD COMPLETELY OVERLAP THE HORIZONTAL STRIPS DUE TO MATERIAL MANUFACTURING SHALL BE AT LEAST ONE FOOT IN LENGTH, IF NOT VULCANIZED OR ADHESIVED, OR 6 INCHES IN LENGTH IF THE LAP IS VULCANIZED OR ADHESIVED. NO LAPS ARE ACCEPTABLE IN VERTICALLY INSTALLED NEOPRENE STRIPS.

PAYMENT FOR LABOR, MATERIALS AND INSTALLATION OF THESE ITEMS SHALL BE INCLUDED IN ITEM 842 CLASS C CONCRETE, ABUTMENT, AS PER PLAN.

ITEM 503, UNCLASSIFIED EXCAVATION ***, AS PER PLAN

UNCLASSIFIED EXCAVATION SHALL BE IN ACCORDANCE WITH 503 EXCEPT THAT THE BACKFILL MATERIAL BEHIND THE ABUTMENTS SHALL BE 203 MATERIAL PLACED IN 6 INCH LIFTS.

DESIGN AGENCY
EMERSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
SP	SP	CEE	2-24-99
CHECKED	REVISED		
RT			

STRUCTURE FILE NO. 2400448

GENERAL NOTES
BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

3 / 16

469
522

ESTIMATED QUANTITIES

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	LEFT BRIDGE ABUT.	LEFT BRIDGE PIER	LEFT BRIDGE SUPER.	RIGHT BRIDGE ABUT.	RIGHT BRIDGE PIER	RIGHT BRIDGE SUPER.	GEN.
503	21100	1190	CU YD	UNCLASSIFIED EXCAVATION	432	163		432	163		
505	11100	LUMP	LUMP	PILE DRIVING EQUIPMENT MOBILIZATION							LUMP
507	00500	4680	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, DRIVEN	1540	800		1540	800		
507	00550	5440	LIN.FT.	12" CAST-IN-PLACE REINFORCED CONCRETE PILES, FURNISHED	1760	960		1760	960		
507	50500	76	EACH	STEEL PILE SPLICES	22	16		22	16		
SPECIAL	51267500	1324	SQ YD	SEALING OF CONCRETE SURFACES *	120	597	607	120	597	607	
516	13700	110	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	55			55			
516	44001	28	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.	14			14			
				7 1/2" x 11" x 1 1/2" NEOPRENE WITH 1 1/2" x 8 1/2" x 12" STEEL							
				LOAD PLATE, AS PER PLAN *							
516	44001	14	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.		7			7		
				10" x 17" x 1 1/2" NEOPRENE WITH 1 1/2" x 11" x 24" STEEL							
				LOAD PLATE, AS PER PLAN *							
516	44001	14	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE.		7			7		
				10" x 17" x 1 1/2" NEOPRENE WITH 1 1/2" x 11" x 16" STEEL							
				LOAD PLATE, AS PER PLAN *							
518	21200	127	CU YD	POROUS BACKFILL WITH FILTER FABRIC	64			64			
518	40000	432	LIN FT	6" PERFORATED CORRUGATED PLASTIC PIPE	216			216			
518	40010	320	LIN FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE,	160			160			
				INCLUDING SPECIALS							
816	00600	LUMP	SUM	FIELD PAINTING OF NEW STEEL, INTERMEDIATE AND FINISH COAT,			LUMP			LUMP	
				SYSTEM IZEU.							
844	48000	596	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (DECK)			298			298	
844	48020	106	CU YD	HIGH PERFORMANCE CONCRETE, SUPERSTRUCTURE (PARAPET)			53			53	
844	49000		LUMP	HIGH PERFORMANCE CONCRETE, TRIAL MIX							
844	49010		LUMP	HIGH PERFORMANCE CONCRETE, TESTING							
842	41000	187	CU YD	CLASS C CONCRETE, PIER CAPS AND COLUMNS		94			94		
842	44101	231	CU YD	CLASS C CONCRETE, ABUTMENT NOT INCLUDING FOOTING,	116			116			
				AS PER PLAN							
842	46500	425	CU YD	CLASS C CONCRETE, FOOTING	146	67		146	67		
863	10040	LUMP	SUM	STRUCTURAL STEEL MEMBERS, LEVEL TWO (2) FABRICATION			LUMP			LUMP	
863	20000	6342	EACH	WELDED STUD SHEAR CONNECTOR			3171			3171	
				Computed by: S.P. Date: 6-8-94							
				Checked by: R.T. Date: 7-8-94							

* SEE PROPOSAL NOTE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

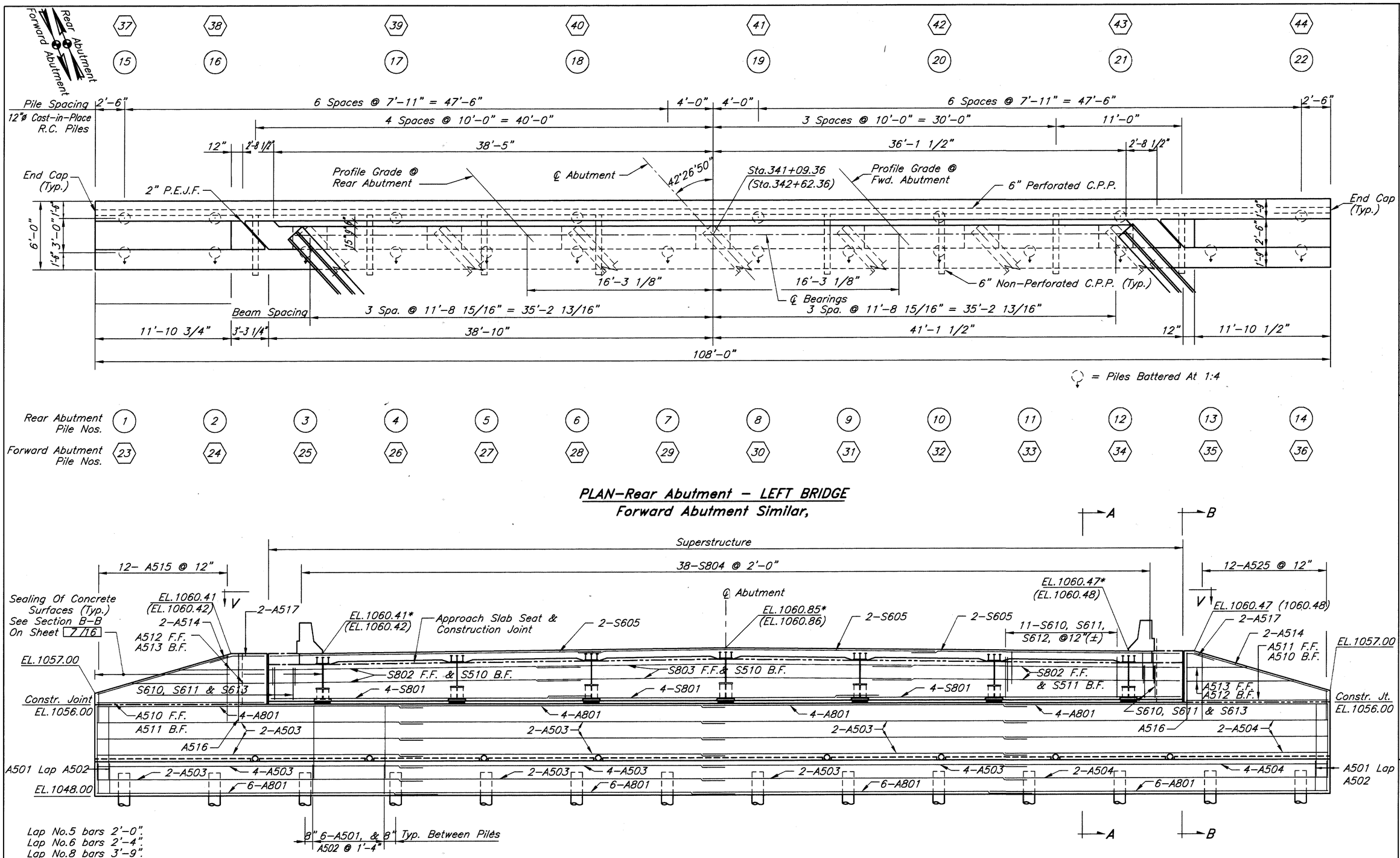
CHECKED VK | REVISED

STRUCTURE FILE NO.2400448

ESTIMATED QUANTITIES
BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

Revised 8/11/00 by E.R.P.



PLAN–Rear Abutment – LEFT BRIDGE
Forward Abutment Similar,

Lap No.5 bars 2'–0".
Lap No.6 bars 2'–4".
Lap No.8 bars 3'–9".

ABBREVIATIONS
F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Polyethylene Pipe

ELEVATION

* Elevations are given @ ℄ Bearings

Elevations & Station in ( ) are for Forward Abutment only.

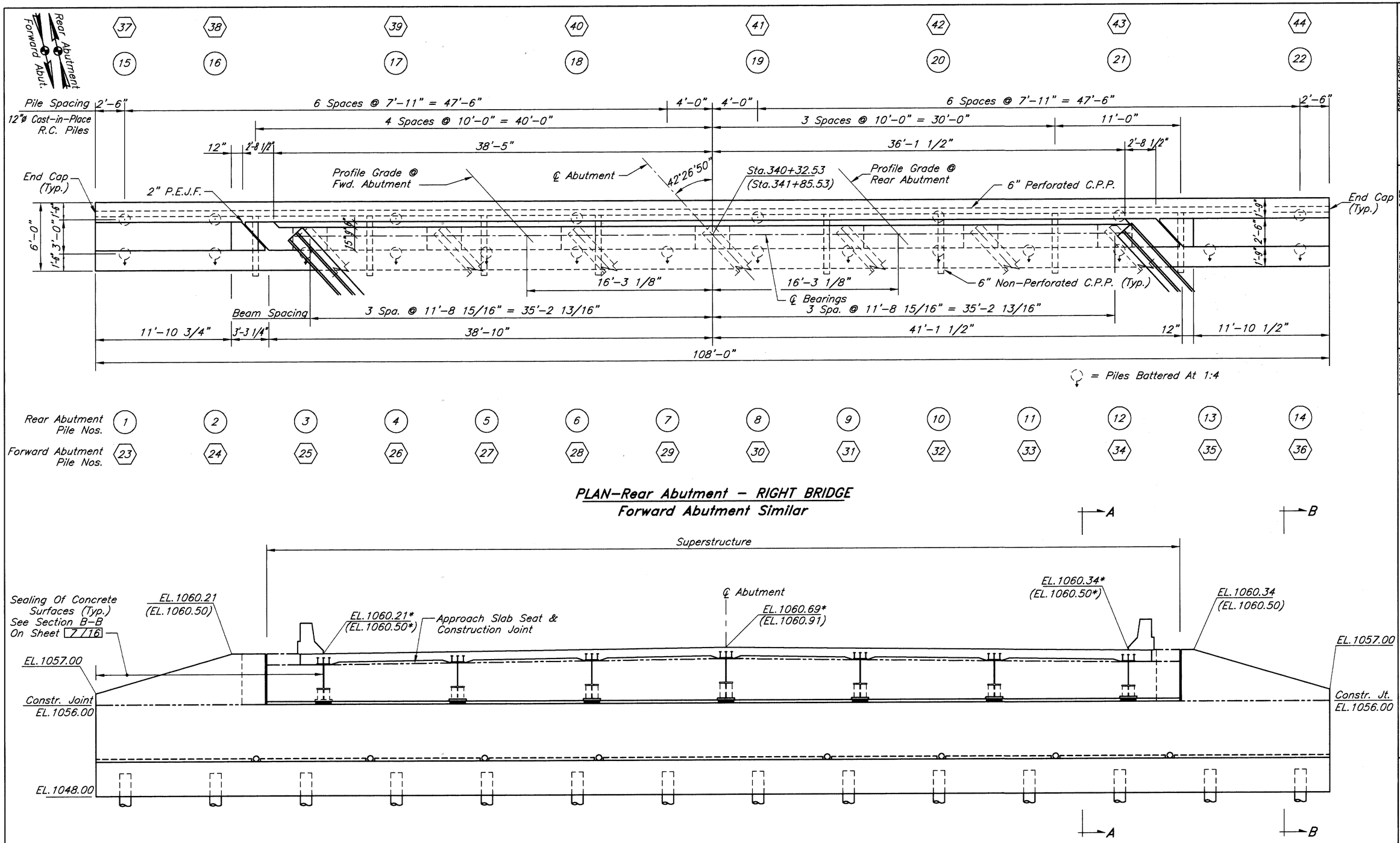
For notes and Sections A–A , B–B & V–V see sheet 7/16

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED SP/VK | REVISED | STRUCTURE FILE NO.2400448

ABUTMENT DETAILS
Bridge No. FAY-35-0644 (Left)
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57



PLAN–Rear Abutment – RIGHT BRIDGE
Forward Abutment Similar

ELEVATION

ABBREVIATIONS
F.F. = Front Face
B.F. = Back Face
E.F. = Each Face
C.P.P. = Corrugated Polyethylene Pipe

* Elevations are given @ ℄ Bearings

Elevations & Station in ( ) are for Forward Abutment only.

For notes and Sections A–A & B–B, see sheet 7/16

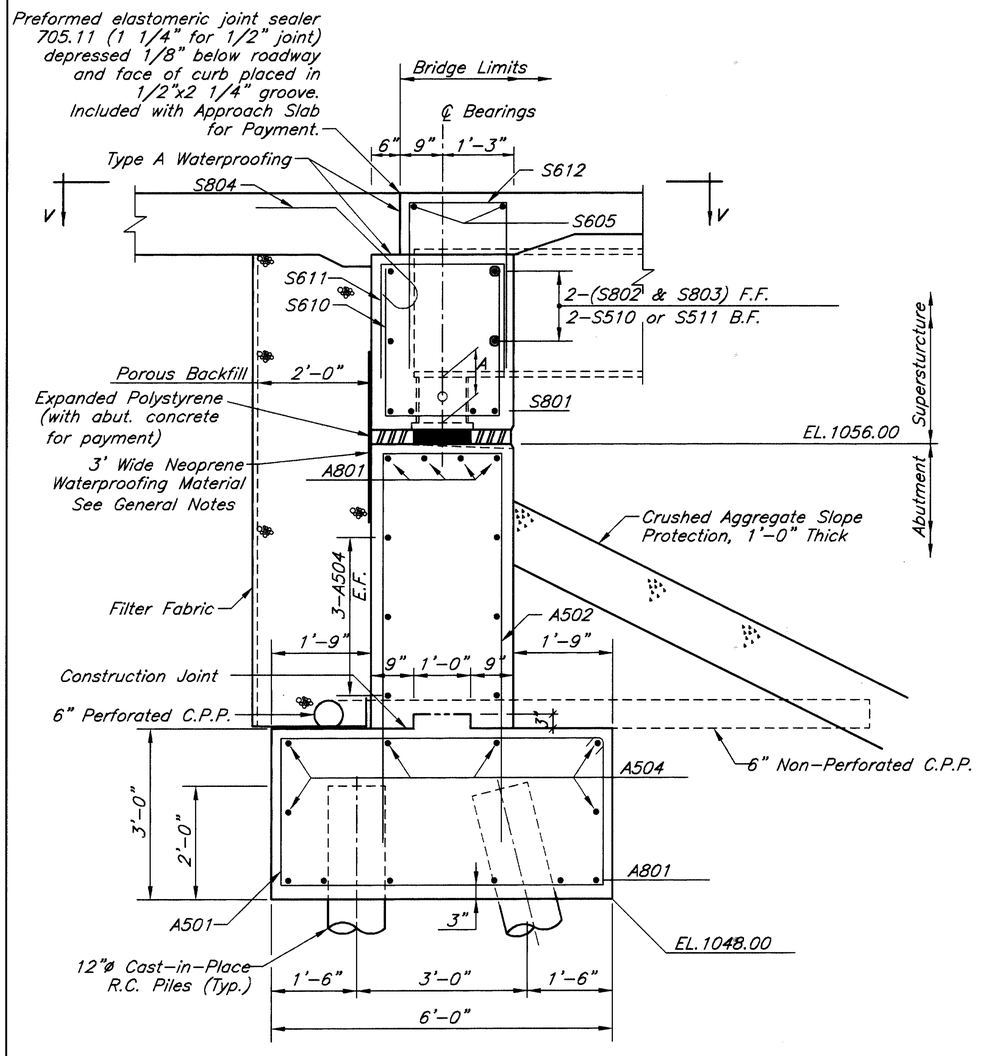
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO.2400448

ABUTMENT DETAILS
Bridge No. FAY-35-0644 (Right)
US 35 OVER MILLEDGEVILLE-OCTA ROAD

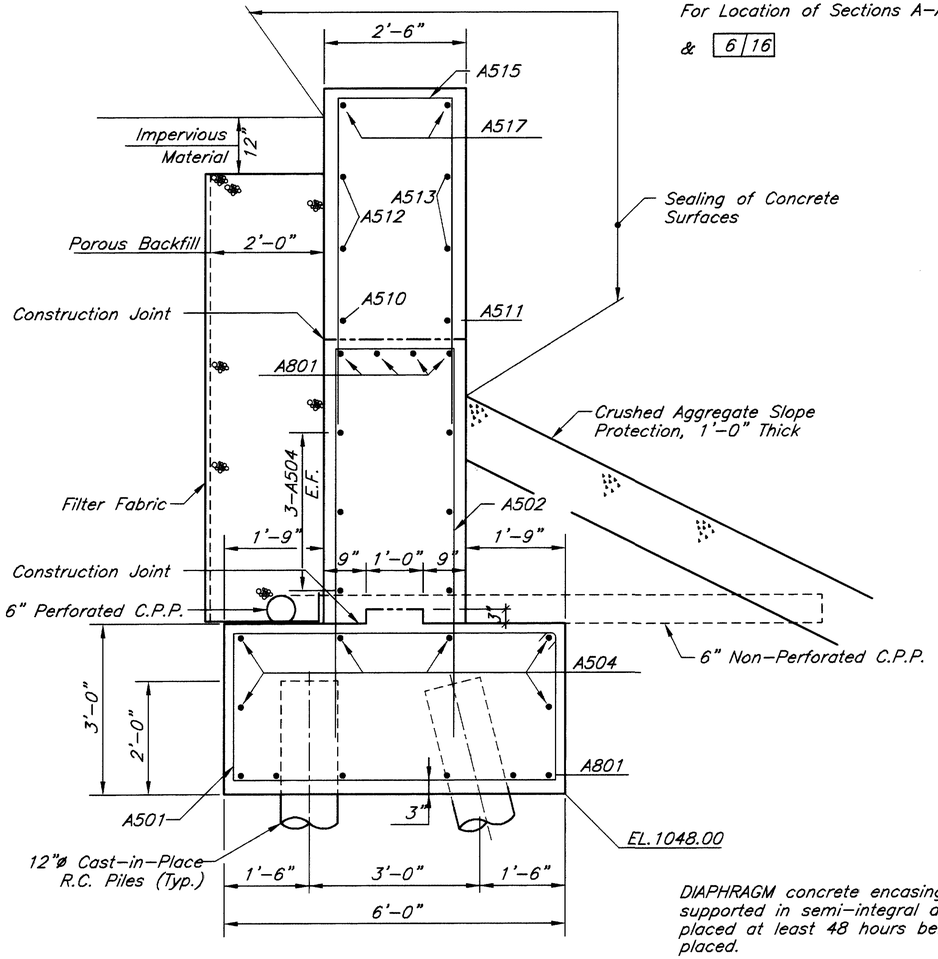
FAY-35-2.57



SECTION A-A

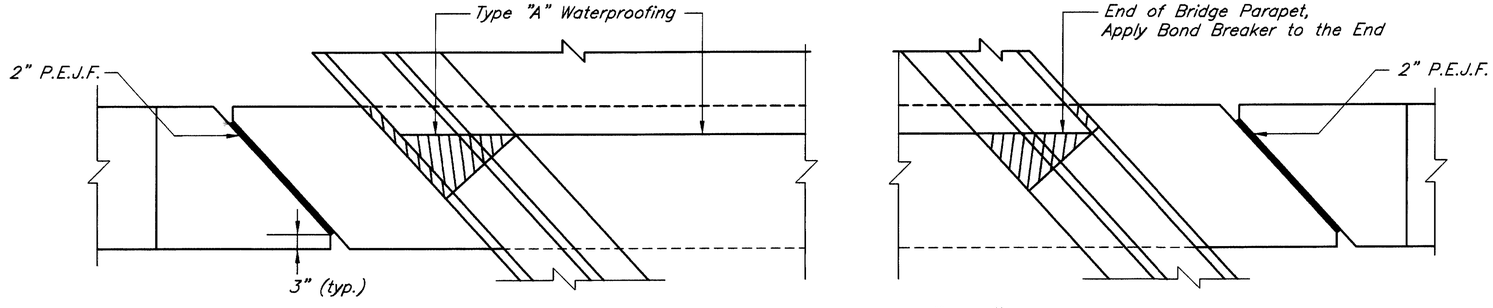
POROUS BACKFILL with filter fabric shall extend upward to the plane of the subgrade,to one foot below the embankment surface, and laterally to the ends of the wingwalls. Geotextile fabric shall conform with 712.09, Type A. Geotextile fabric is included with porous backfill for payment.

For Location of Sections A-A & B-B see sheet 5|16 & 6|16



SECTION B-B

DIAPHRAGM concrete encasing the structural member sections supported in semi-integral and integral type abutments shall be placed at least 48 hours before the actual deck concrete is placed.



PORTION OF VIEW "V-V"

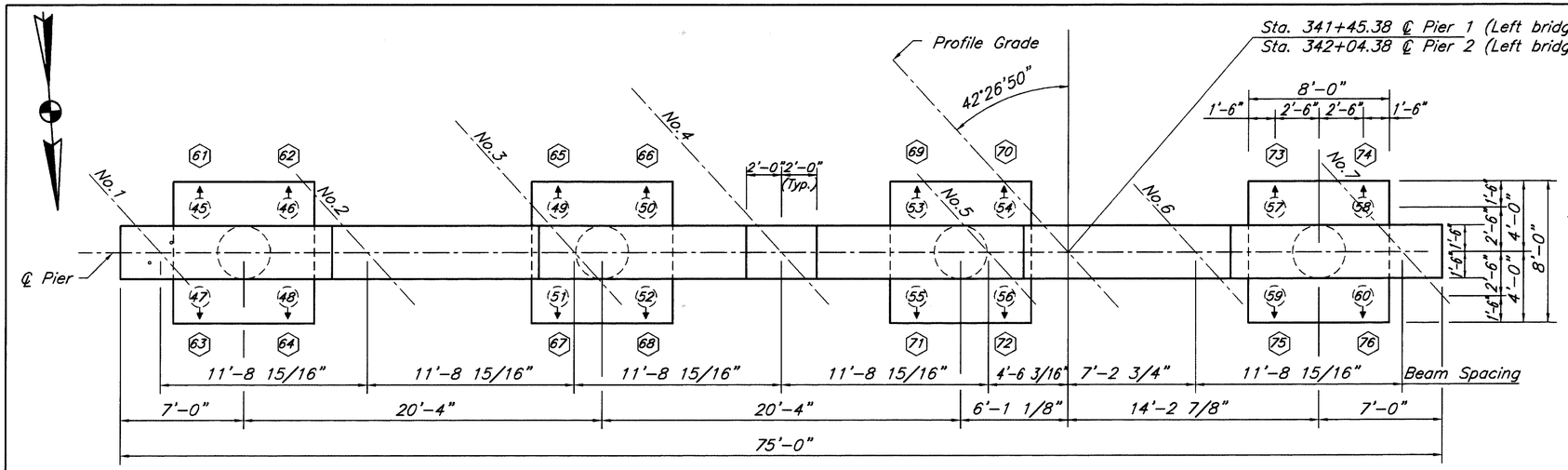
DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400448

ABUTMENT DETAILS
Bridge No. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

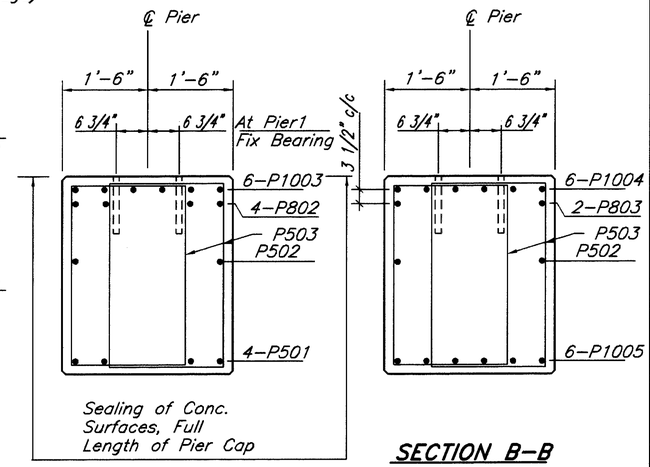
FAY-35-2.57



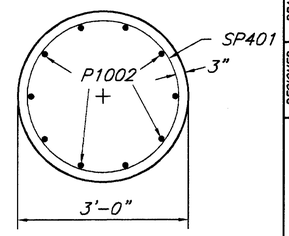
PLAN – LEFT BRIDGE PIER

NOTES:

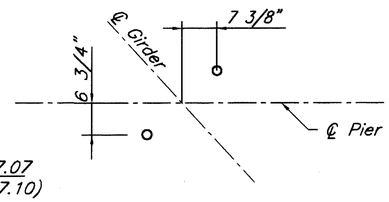
Footing & Pile Dimensions are Typical for All Footings.
Numbers in ⬡ are for Pier 2 piles.
(55) Indicate Piles Battered at 1:4.



SECTION A–A

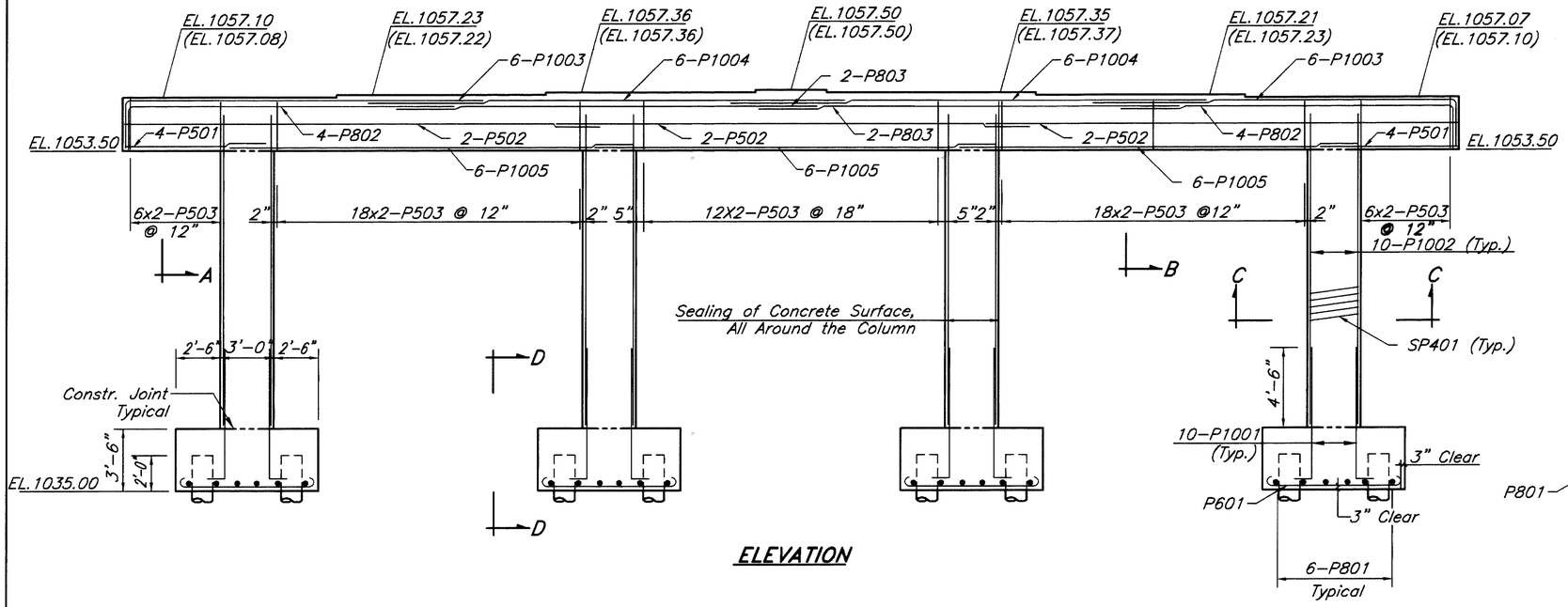


SECTION B–B

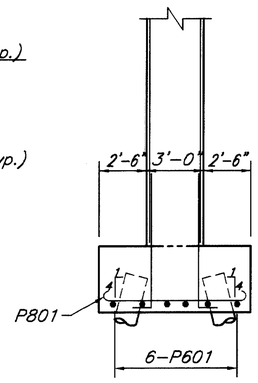


BEARING ANCHORS LAYOUT

SECTION C–C



ELEVATION



VIEW D–D

Elevation in ( ) are for Pier 2.

BRIDGE SEAT REINFORCING:

REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT SHALL BE ACCURATELY PLACED TO AVOID INTERFERENCE WITH THE DRILLING OF BEARING ANCHOR HOLES OR THE PRE–SETTING OF BEARING ANCHORS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

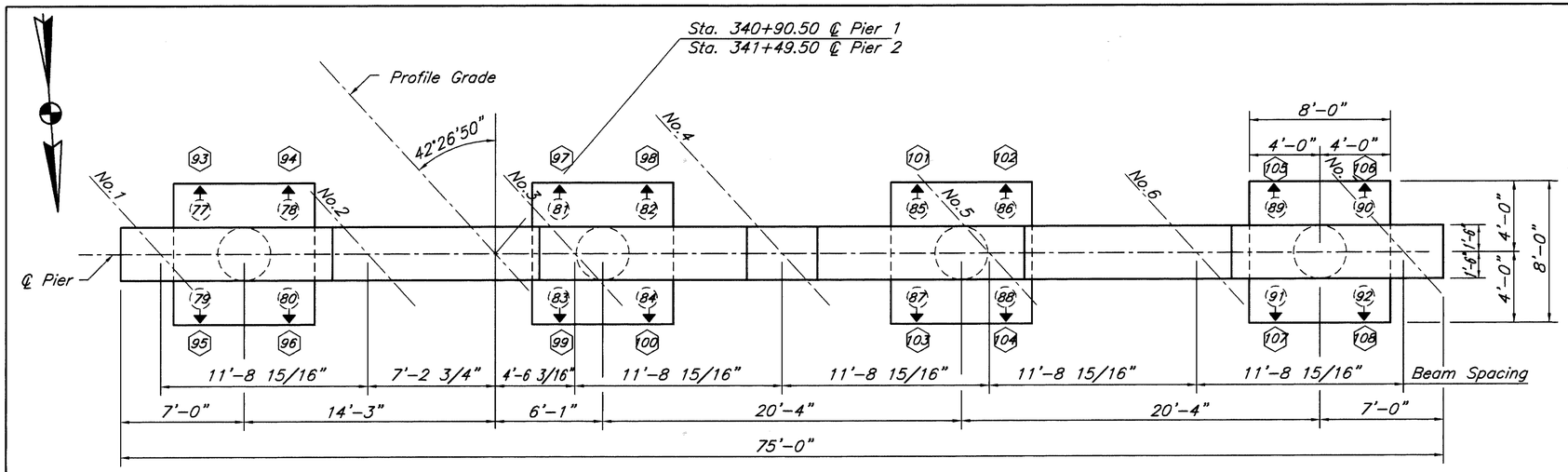
DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVIEWED CEE
DATE 2-24-99
REVISED
STRUCTURE FILE NO.2400448

PIER DETAILS
Bridge No. FAY-35-0644 (LEFT)
US 35 OVER MILLEDGEVILLE OCTA-ROAD

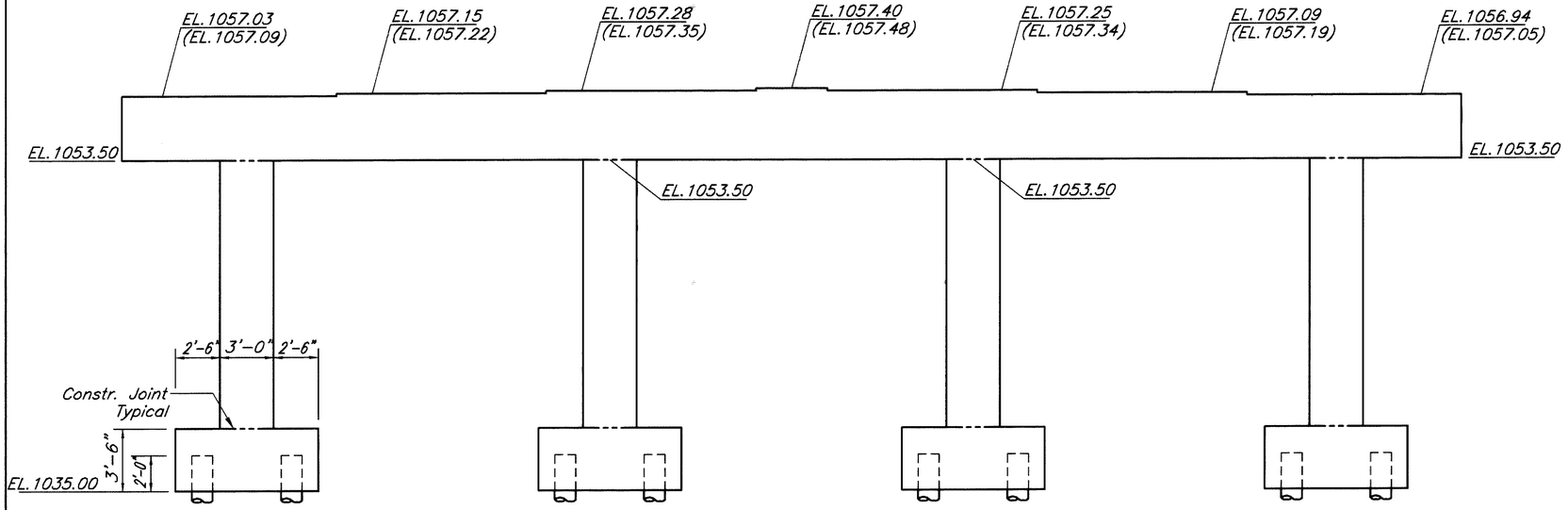
FAY-35-2.57

8 / 16

474
522



PLAN – RIGHT BRIDGE PIER



ELEVATION

Notes:

Elevations in ( ) are for Pier 2.

For reinforcing details see sheet 8 | 16

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

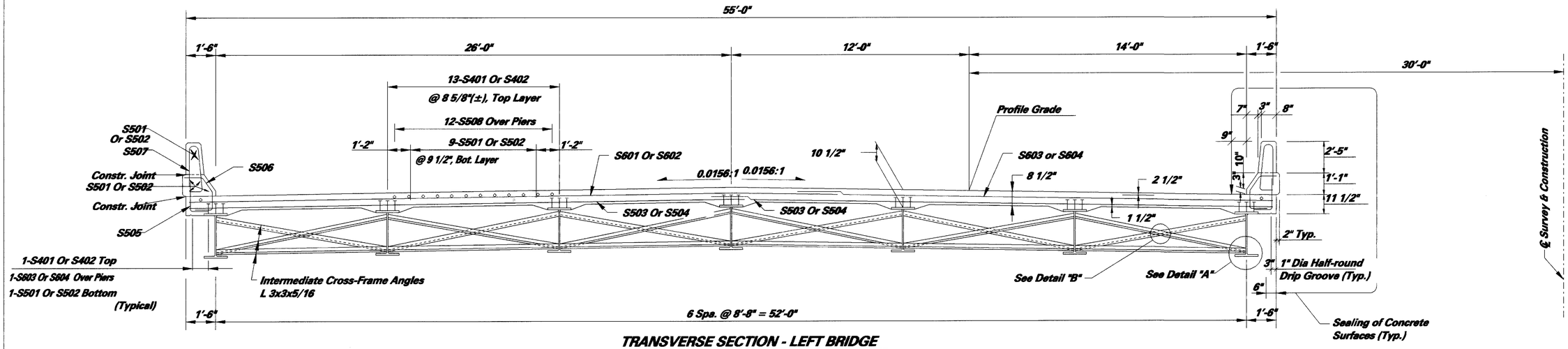
STRUCTURE FILE NO. 2400448

PIER DETAILS
Bridge No. FAY-35-0644 (RIGHT)
US 35 OVER MILLEDGEVILLE OCTA-ROAD

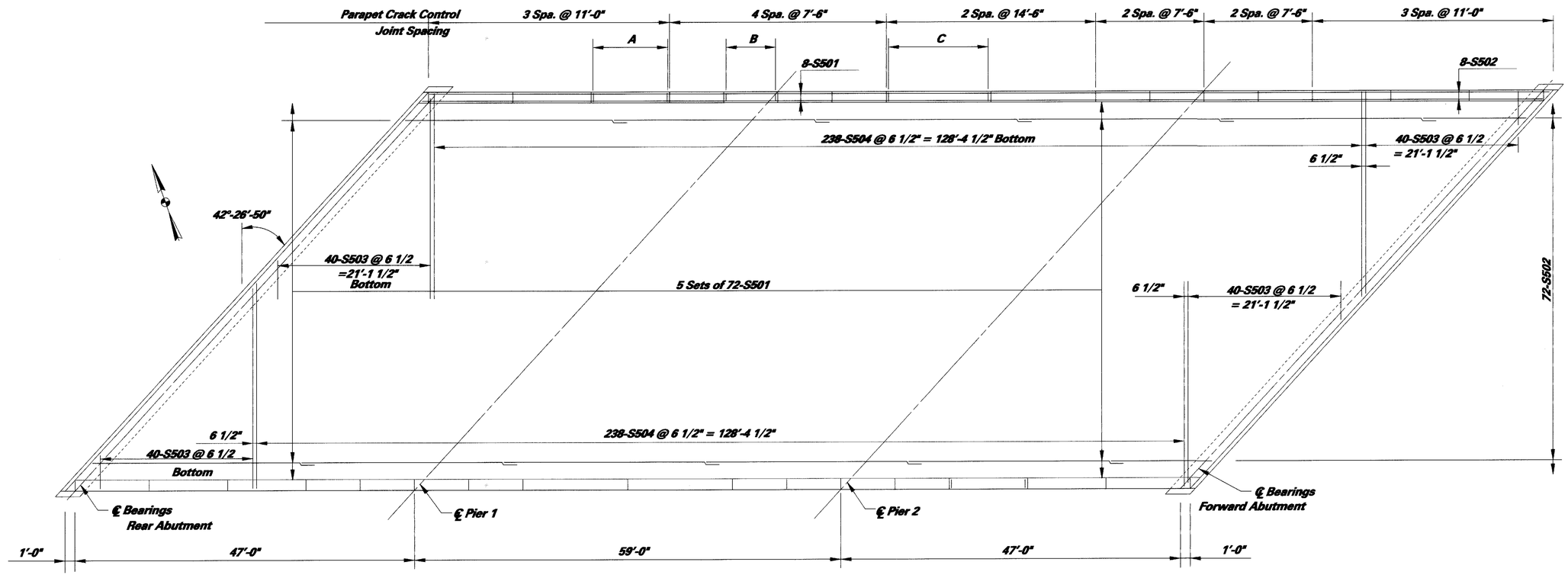
FAY-35-2.57

9 | 16

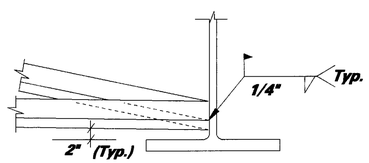
475
522



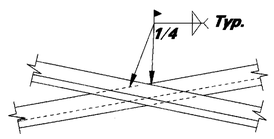
TRANSVERSE SECTION - LEFT BRIDGE
Similar for Right Bridge



DECK REINFORCING PLAN-LEFT & RIGHT BRIDGES
(Bottom of slab & parapet reinforcement)



DETAIL "A"



DETAIL "B"

A = 9-S505, S506 & S507 @ 1'-4"

B = 6-S505, S506 & S507 @ 1'-5"

C = 11-S505, S506 & S507 @ 1'-5"

Lap No.4 bars 1'-8" min.
No.5 bars 2'-0" min.
No.6 bars 2'-4" min.

For Notes See Sheet 12/16

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

REVIEWED	DATE
CEE	2-24-99

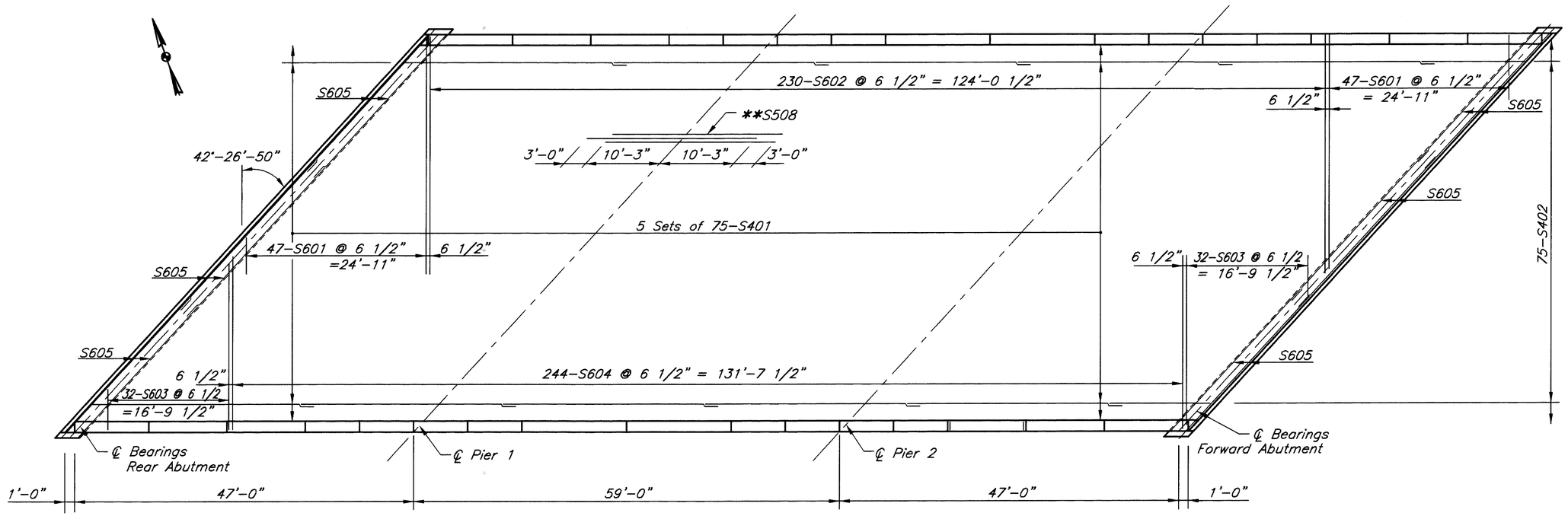
STRUCTURE FILE NO. 2400449

DRAWN	REVISED
RT	

DESIGNED	CHECKED
RT	SP/VK

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY-35-0844
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.56



DECK REINFORCING PLAN–LEFT & RIGHT BRIDGES

(Top reinforcement shown)

Lap No.4 bars 1'–8" min.
No.5 bars 2'–0" min.
No.6 bars 2'–4" min.

** 74–S508 required over each pier

For Notes See Sheet 12/16

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2–24–99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400448

SUPERSTRUCTURE DETAILS
BRIDGE NO. FAY–35–0644
US 35 OVER MILLEDGEVILLE–OCTA ROAD

FAY–35–2.56

SCREED ELEVATION TABLE - RIGHT BRIDGE

		SCREED LINE 1		SCREED LINE 2		SCREED LINE 3		SCREED LINE 4		SCREED LINE 5		SCREED LINE 6		SCREED LINE 7	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	340+56.31	1060.35	340+48.38	1060.46	340+40.46	1060.58	340+32.53	1060.69	340+24.60	1060.53	340+16.68	1060.37	340+8.75	1060.21
	0.50 L	340+79.81	1060.43	340+71.88	1060.55	340+63.96	1060.66	340+56.03	1060.78	340+48.10	1060.63	340+40.18	1060.47	340+32.25	1060.31
	1.00 L	341+3.31	1060.44	340+95.38	1060.56	340+87.46	1060.68	340+79.53	1060.80	340+71.60	1060.65	340+63.68	1060.50	340+55.75	1060.35
Span No.2	0.25 L	341+18.06	1060.48	341+10.13	1060.60	341+2.21	1060.73	340+94.28	1060.85	340+86.35	1060.70	340+78.43	1060.55	340+70.50	1060.40
	0.50 L	341+32.81	1060.50	341+24.88	1060.63	341+16.96	1060.75	341+9.03	1060.88	341+1.10	1060.73	340+93.18	1060.58	340+85.25	1060.43
	0.75 L	341+47.56	1060.51	341+39.63	1060.64	341+31.71	1060.76	341+23.78	1060.89	341+15.85	1060.75	341+7.93	1060.60	340+100.00	1060.45
	1.00 L	341+62.31	1060.50	341+54.38	1060.63	341+46.46	1060.76	341+38.53	1060.89	341+30.60	1060.75	341+22.68	1060.60	341+14.75	1060.46
Span No.3	0.50 L	341+85.81	1060.53	341+77.88	1060.67	341+69.96	1060.80	341+62.03	1060.93	341+54.10	1060.79	341+46.18	1060.65	341+38.25	1060.51
	1.00 L	342+9.31	1060.50	342+1.38	1060.64	341+93.46	1060.78	341+85.53	1060.91	341+77.60	1060.77	341+69.68	1060.64	341+61.75	1060.50

SCREED ELEVATIONS are for the concrete surface before the concrete is placed. Allowance has been made for the anticipated deflection due to weight of Deck Concrete.

L = Span Length

CONCRETE DECK SLAB DEPTH

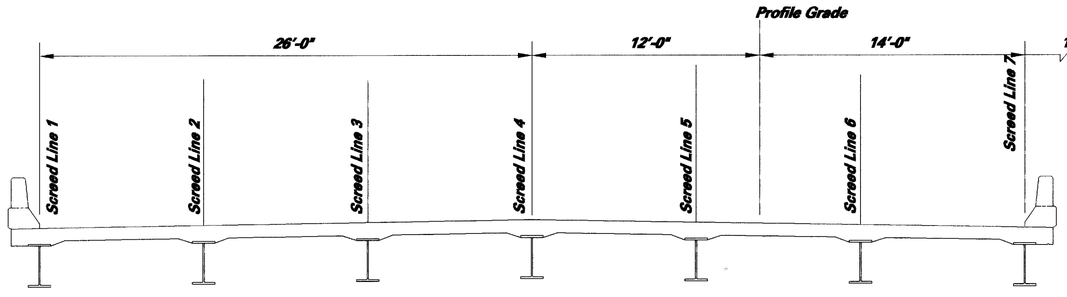
The distance shown from the top of the deck slab to the top of the flange is the theoretical design dimension. The quantity of the deck concrete to be paid for shall be based upon this dimension even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade. Deduction shall be made for volume of encased steel flanges as per 842.

CONCRETE DECK HAUNCH WIDTH

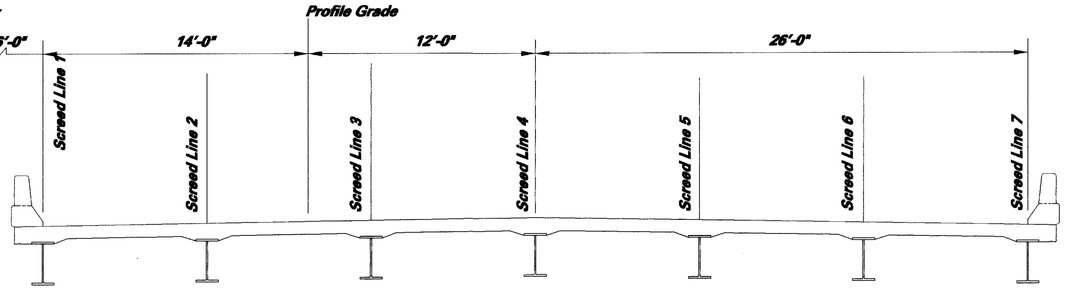
A haunch width of 9" shall be used for computing quantity of the deck concrete. However, the haunch width may vary between 6 and 12 inches.

SCREED ELEVATION TABLE - LEFT BRIDGE

		SCREED LINE 1		SCREED LINE 2		SCREED LINE 3		SCREED LINE 4		SCREED LINE 5		SCREED LINE 6		SCREED LINE 7	
SPAN NO.	LOCATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION	STATION	SCREED ELEVATION
Span No.1	0.00 L	341+33.13	1060.48	341+25.20	1060.60	341+17.28	1060.73	341+09.35	1060.85	341+01.42	1060.71	340+93.50	1060.56	340+85.57	1060.41
	0.50 L	341+56.63	1060.52	341+48.70	1060.65	341+40.78	1060.78	341+32.85	1060.91	341+24.92	1060.77	341+17.00	1060.62	341+09.07	1060.48
	1.00 L	341+80.13	1060.50	341+72.20	1060.64	341+64.28	1060.77	341+56.35	1060.90	341+48.42	1060.76	341+40.50	1060.62	341+32.57	1060.48
Span No.2	0.25 L	341+94.88	1060.52	341+86.95	1060.66	341+79.03	1060.79	341+71.10	1060.93	341+63.17	1060.79	341+55.25	1060.65	341+47.32	1060.51
	0.50 L	342+09.63	1060.53	342+01.70	1060.66	341+93.78	1060.80	341+85.85	1060.94	341+77.92	1060.80	341+70.00	1060.66	341+62.07	1060.52
	0.75 L	342+24.38	1060.51	342+16.45	1060.65	342+08.53	1060.79	342+00.60	1060.93	341+92.67	1060.79	341+84.75	1060.66	341+76.82	1060.52
	1.00 L	342+39.13	1060.48	342+31.20	1060.62	342+23.28	1060.77	342+15.35	1060.90	342+07.42	1060.77	341+99.50	1060.64	341+91.57	1060.51
Span No.3	0.50 L	342+62.63	1060.48	342+54.70	1060.63	342+46.78	1060.77	342+38.85	1060.92	342+30.92	1060.79	342+23.00	1060.66	342+15.07	1060.53
	1.00 L	342+86.13	1060.42	342+78.20	1060.57	342+70.28	1060.72	342+62.35	1060.86	342+54.42	1060.74	342+46.50	1060.61	342+38.57	1060.48



SCREED LINE LOCATIONS - LEFT BRIDGE



SCREED LINE LOCATIONS - RIGHT BRIDGE

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1623 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99

STRUCTURE FILE NO.2400448

REVISED

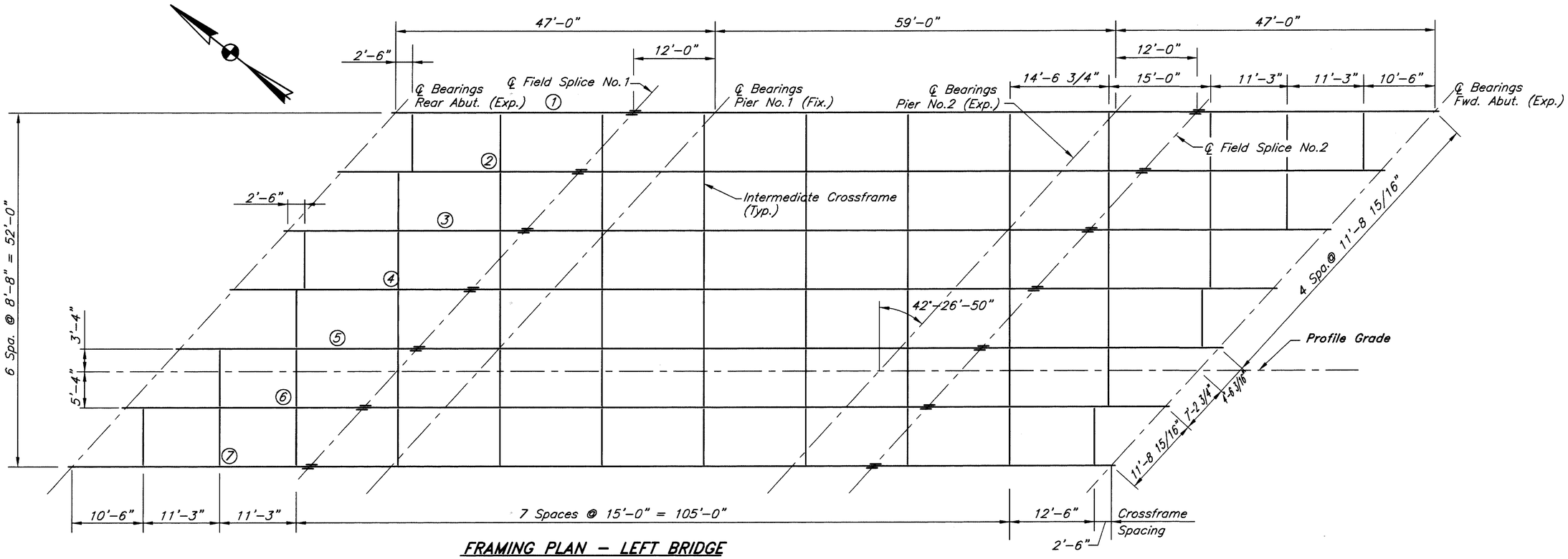
CHECKED SP/VK

SCREED ELEVATIONS
Bridge No. FAY-35-0844
US 35 Over Molledgeville-Octa Road

FAY-35-2.57

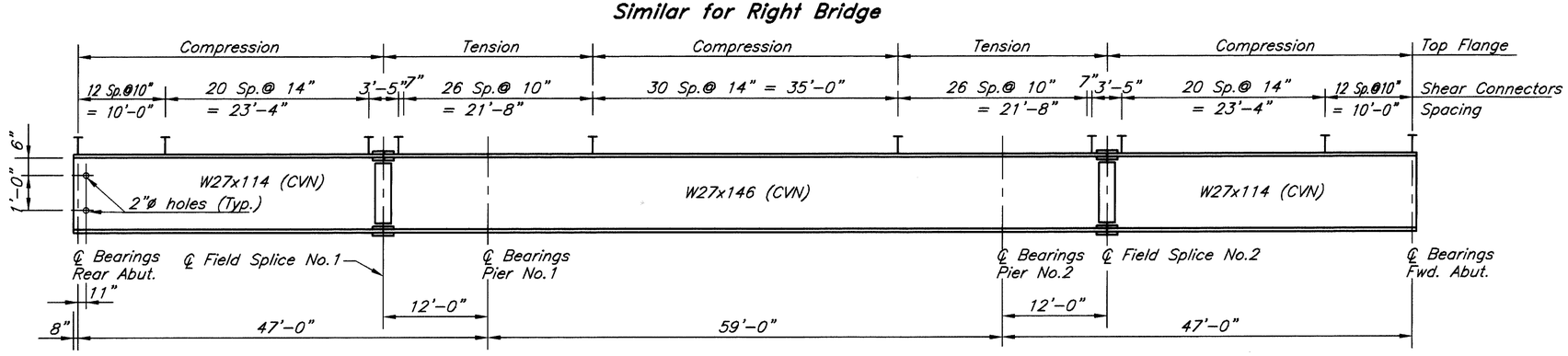
12 / 16

478
522



FRAMING PLAN – LEFT BRIDGE

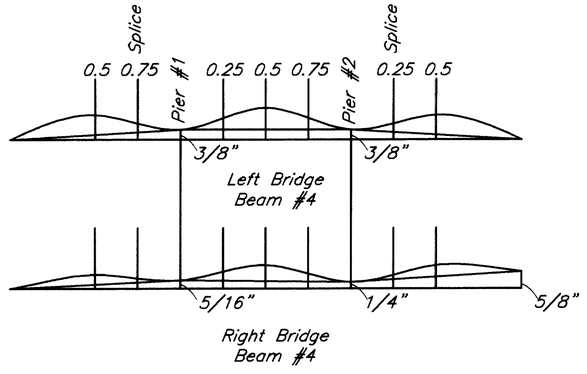
Similar for Right Bridge



BEAM ELEVATION

SHEAR CONNECTOR DETAIL

DEFLECTION AND CAMBER								
	Points	Span No.1		Span No.2			Span No.3	
		0.50	0.75	0.25	0.50	0.75	0.25	0.50
INTERIOR BEAMS	Deflection due to weight of steel	1/16	1/16	1/16	1/16	1/16	1/16	1/16
	Deflection due to remaining dead load	3/8	3/16	3/16	5/16	3/16	3/16	3/8
	Adjustment for Vertical curve	1/16	1/16	1/16	1/16	0	0	0
	Net Total Camber	1/2"	5/16"	5/16"	7/16"	1/4"	1/4"	7/16"



CAMBER DIAGRAM

NOTES :

WELDED ATTACHMENT *of supports for concrete deck finishing machine may be made to areas of the flanges designated "Compression". Attachments shall not be made to areas designated "Tension". Fillet welds to compression flanges shall be not closer than 1" from edge of the flange, be not more than 2" long and be not smaller than the minimum size required by AASHTO.*

CHARPY V-NOTCH TOUGHNESS REQUIREMENT

Where a shape or plate is designated (CVN), the material shall meet minimum notch toughness requirements as specified in 711.01 of CMS.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED	DRAWN	REVIEWED	DATE
RT	RT	CEE	2-24-99
CHECKED	REVISED		
SP/VK			

STRUCTURE FILE NO. 2400448

SUPERSTRUCTURE DETAILS

BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

13 | 16

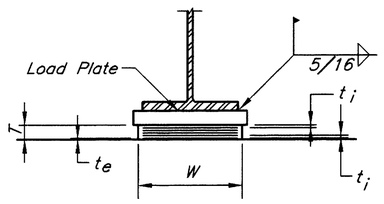
479
522

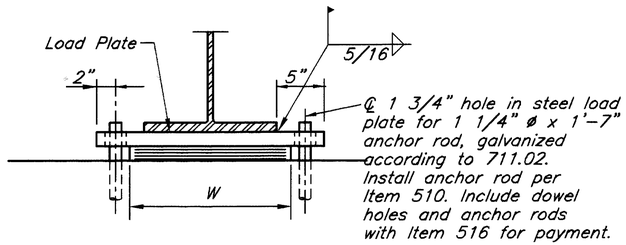
LAMINATED ELASTOMERIC BEARINGS

LOCATION	DIMENSIONS (Inch)					N	STEEL LOAD PLATE	REACTIONS		MAXIMUM DESIGN LOAD
	L	W	t_i	t_e	T			DL	LL	
Abutments Expansion Brngs.	7 1/2	11	0.197	0.141	1 1/2	5	8 1/2" x 12" x 1 1/2"	29 k	47 k	76 k
Pier No.1 Fixed Brngs.	10	17	0.255	0.182	1 1/2	4	11" x 24" x 1 1/2"	104 k	56 k	160 k
Pier No.2 Expansion Brngs.	10	17	0.255	0.182	1 1/2	4	11" x 18" x 1 1/2"	104 k	56 k	160 k

t_i = Thickness of Internal Layer
t_e = Thickness of External Layer
T = Total Thickness of Elastomeric Bearing

N = No. of Steel Laminates
Internal Steel Laminate Thickness = 0.0747"
Durometer of Elastomer = 50 Durometer





LAMINATED ELASTOMERIC EXPANSION BEARING

LOAD PLATE

The steel load plate shall be the same material as the attached structural steel and be similarly cleaned and coated. Surface preparation and priming shall be done in the shop and be included in the price bid for the bearings. Field coats shall be included in the price bid for painting main structural steel.

The steel load plate shall be bonded by vulcanization to the elastomer during the molding process.

Welding of the load plate to the superstructure shall be controlled so that the plate temperature at the elastomer bonded surface shall not exceed 300° F as determined by the use of pyrometric sticks or other temperature monitoring devices.

BASIS OF PAYMENT

The unit bid price shall include all materials, labor and incidentals necessary to furnish and install laminated elastomeric bearings either fixed or expansion. Payment will be made at the contract price for Item 516, each, Laminated Elastomeric Bearings.

LAMINATED ELASTOMERIC FIXED BEARING

ELASTOMERIC BEARINGS

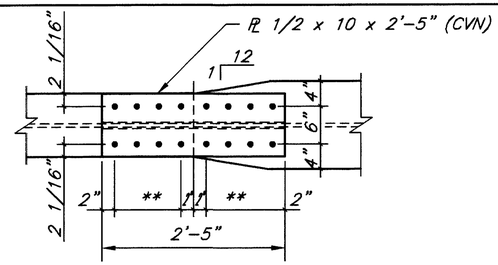
Elastomeric bearings shall comply with Item 516 and articles 18.2.5 through 18.2.8, Section 18, Bearing Devices, Division II, Construction, of the AASHTO Specifications for Highway Bridges. Bearings shall be Grade 3, 50-durometer elastomer and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the price bid for the bearings.

HIGH STRENGTH BOLTS

High strength bolts shall be 1" diameter A325 unless otherwise noted.

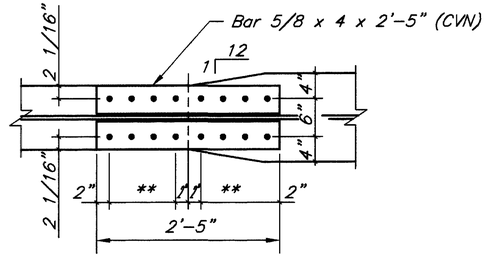
CHARPY V-NOTCH TOUGHNESS REQUIREMENT

Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in 711.01 of CMS.

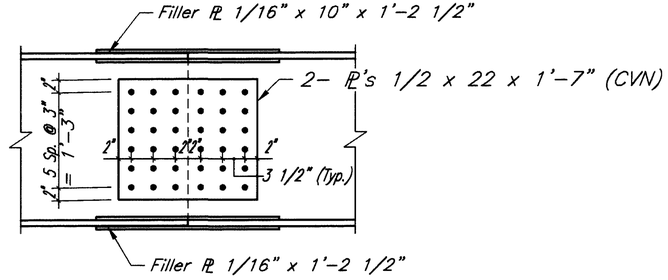


PLAN - TOP FLANGE

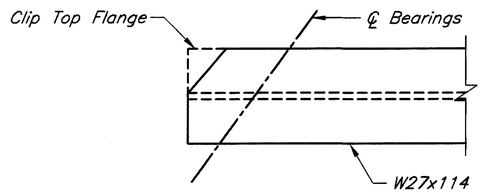
** = 3 Spaces @ 3 1/2" = 10 1/2"



PLAN - BOTTOM FLANGE



TYPICAL FIELD SPLICE DETAILS



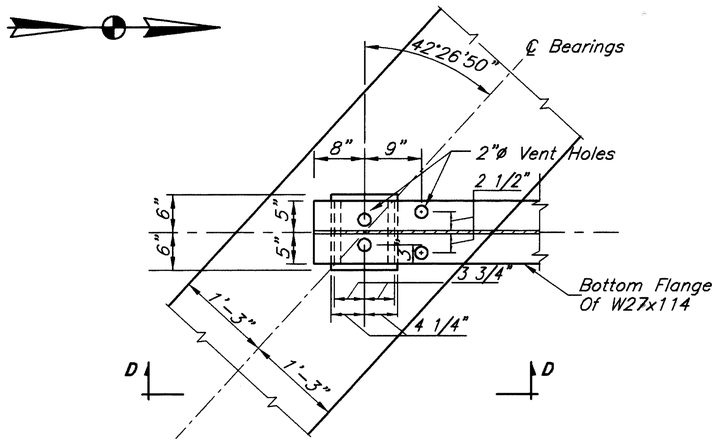
TOP FLANGE DETAIL AT ABUTMENTS

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

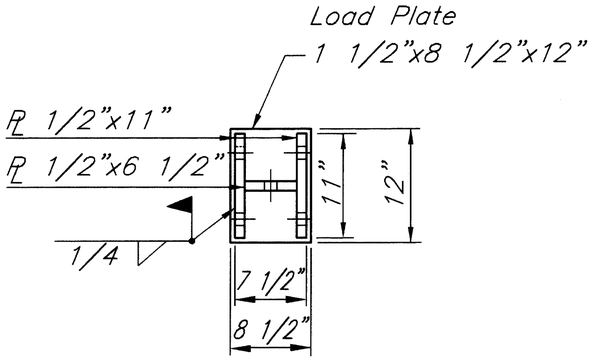
DESIGNED RT
DRAWN RT
CHECKED SP/VK
REVIEWED CEE
DATE 2-24-99
REVISED
STRUCTURE FILE NO.2400448

BEARING & SPLICE DETAILS
Bridge No. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

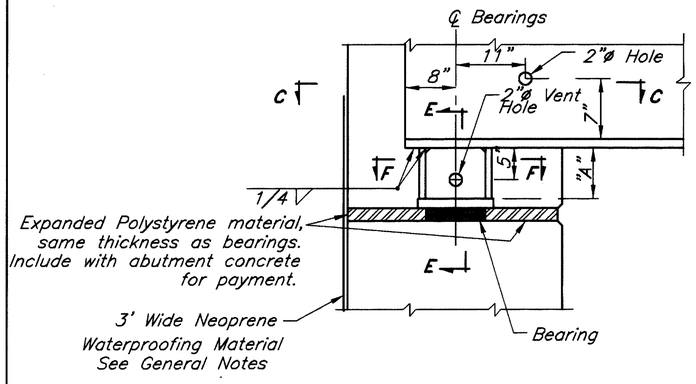
FAY-35-2.57



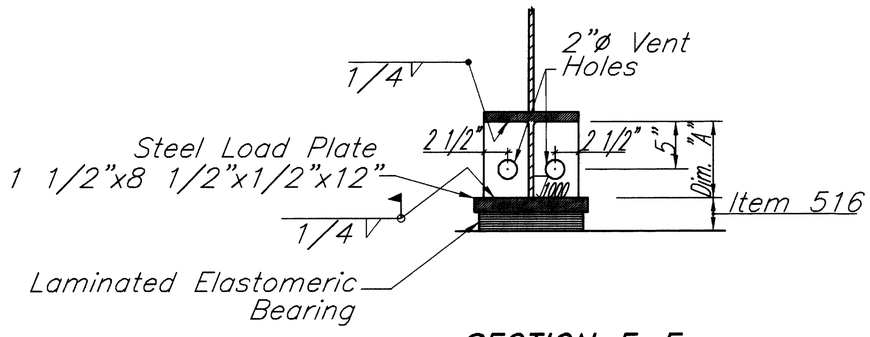
SECTION C–C



SECTION F–F



SECTION D–D



SECTION E–E

BEARING SUPPORT –DIM. "A" LEFT BRIDGE							
BEAM NOS.	**1**	**2**	**3**	**4**	**5**	**6**	**7**
Rear Abutment	1'–1"	1'–2 1/2"	1'–4"	1'–5 1/2"	1'–3 3/4"	1'–1 15/16"	1'–0 1/8"
Forward Abutment	1'–0 1/4"	1'–2 1/16"	1'–3 13/16"	1'–5 5/8"	1'–4 1/16"	1'–2 1/2"	1'–1 1/16"

BEARING SUPPORT –DIM. "A" RIGHT BRIDGE							
BEAM NOS.	**1**	**2**	**3**	**4**	**5**	**6**	**7**
Rear Abutment	11 3/8"	1'–0 7/8"	1'–2 3/16"	1'–3 1/2"	1'–1 11/16"	11 3/4"	9 13/16"
Forward Abutment	1'–1 5/16"	1'–2 7/8"	1'–4 9/16"	1'–6 1/8"	1'–4 9/16"	1'–2 7/8"	1'–1 3/16"

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1523 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED RT | DRAWN RT | REVIEWED CEE | DATE 2-24-99
CHECKED VK | REVISED | STRUCTURE FILE NO. 2400448

BEARING AND JOINT SEAL DETAILS
BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE - OCTA ROAD

FAY-35-2.57

15 | 16

481
522

REINFORCING STEEL LIST

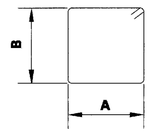
MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				ABUTMENTS (ONE BRIDGE)					
A501	164	17-1	2922	1	5-8	2-7			
A502	164	15-0	2566	2	2-2	6-6 1/2			
A503	72	30-0	2253	STR					
A504	24	23-8	592	STR					
A510	4	14-7	61	STR					
A511	4	12-7	52	STR					
A512	8	7-4 TO	78	STR					4 Sets of 2
		11-4							Vary "B" by 4'-0"
A513	8	5-2 to	60	STR					4 Sets of 2
		9-12							Vary "B" by 4'-0"
A514	8	12-0	100	STR					
A515	48	6-11 to	505	2	2-2	2-6 to			4 Sets of 12
		13-3				5-8			Vary "B" by 3 1/2"
A516	4	13-11	58	2	2-2	6-10			
A517	8	4-6	38	STR					Bend in Field
A801	80	29-9	6355	STR					
			15640	LBS					
				x2 BRIDGES = 31,280 LBS, EPOXY COATED					
				PIERS (ONE BRIDGE)					
P501	16	8-0	134	STR					
P502	12	26-3	329	STR					
P503	240	11-0	2754	1	2-0 1/2	3-2			
P601	48	8-10	637	9	7-6				
P801	48	9-4	1196	9	7-6				
P802	16	21-2	904	4	2-6 1/2	18-10			
P803	8	22-3	475	STR					
P1001	80	8-11	3069	4	1-6	7-9			
P1002	80	17-5	5996	STR					
P1003	24	23-5	2418	4	3-1	20-8			
P1004	24	23-9	2453	STR					
P1005	36	23-9	3679	STR					
Spirals			2304						
			26348	LBS					
				x2 BRIDGES = 52696 LBS, EPOXY COATED					

Dimensions shown are out to out.

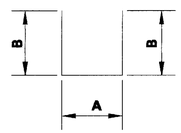
MARK	NO.	LENGTH	WEIGHT	TYPE	A	B	C	D	REMARKS
				SUPERSTRUCTURE (AT ABUTMENT) ONE BRIDGE.					
S510	8	30-0	250	STR					
S511	4	23-6	98	STR					
S610	140	7-0	1472	2	2-2	2-7			
S611	140	6-0	1262	2	2-2	2-1			
S612	132	7-6	1487	2	1-8	3-1			
S613	8	8-9	105	2	2-11	3-1			
S801	24	29-0	1858	STR					
S802	8	27-6	587	STR					
S803	4	32-0	342	STR					
S804	76	5-0	1015	3	2-8	1-5			
			8476	LBS					x2 BRIDGES = 16,952 LBS, EPOXY COATED
				SUPERSTRUCTURE (DECK) ONE BRIDGE.					
S401	375	30-0	7515	STR					
S402	75	13-0	651	STR					
S501	360	30-0	11264	STR					
S502	72	14-9	1108	STR					
S503	160	5-0 to	2747	STR					4 SETS OF 40
		27-11							VARY BY 7"±
S504	476	28-4	14067	STR					
S505	248	2-4	604	4	10 1/2	1-7			
S506	248	3-4	862	5	1-0				
S507	248	7-1	1832	6					
S508	148	23-6	3629	STR					
S601	94	5-7 to	2683	STR					2 SETS OF 47
		32-5							VARY BY 7"
S602	230	33-0	11400	STR					
S603	64	5-4 to	1382	STR					2 SETS OF 32
		23-5							VARY BY 7"
S604	244	24-0	8796	STR					
S605	12	28-0	505	STR					
			69044	LBS					x2 BRIDGES = 137,140 LBS, EPOXY COATED

Pitch 4 1/2"

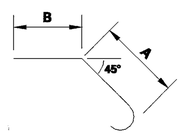
ALL BARS SHALL BE EPOXY COATED



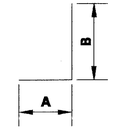
TYPE 1



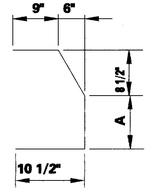
TYPE 2



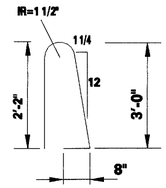
TYPE 3



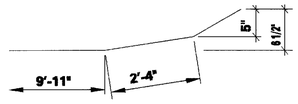
TYPE 4



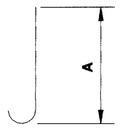
TYPE 5



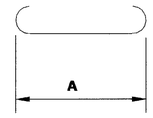
TYPE 6



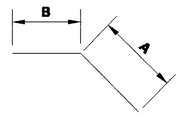
TYPE 7



TYPE 8



TYPE 9



TYPE 10

MARK	NO.	LENGTH	WEIGHT	SHAPE	REMARKS
SP401	8	15-2	2304	BT.	
			2304	LBS.	

SP401 - Core Diameter 30" c/c

Other Details in Accordance with C.R.S.I. Standard Practice.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the length of the spiral along the axis of the spiral. Four steel channel, tee or angle spacers, weighing approximately 0.80 lb. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coils. The number of pounds of these spacers, based on 3.20 per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantities of spiral bars.

DESIGN AGENCY
ERIKSSON ENGINEERING LTD.
1823 CHESAPEAKE AVENUE
COLUMBUS, OH 43212

DESIGNED SP | DRAWN SP | REVIEWED CEE | DATE 2-24-99

CHECKED VK | REVISED

STRUCTURE FILE NO. 2400448

REINFORCING STEEL LIST
BRIDGE NO. FAY-35-0644
US 35 OVER MILLEDGEVILLE-OCTA ROAD

FAY-35-2.57

SCANNED
BY: Dale Mead
DATE: 6-5-00
06r1027

FAY-35-2.57

V.M.S. 1343, 1361 & 1223
JEFFERSON TOWNSHIP
V.M.S. 1278
JASPER TOWNSHIP
FAYETTE COUNTY

RECEIVED _____, 19____
RECORDED _____, 19____
PLAT BOOK _____ PAGE _____
COUNTY RECORDER

SCALE IN FEET: 0, 250, 500, 1000

P I D NO. 9078

BASIS FOR BEARING:
STATE PLANE COORDINATES

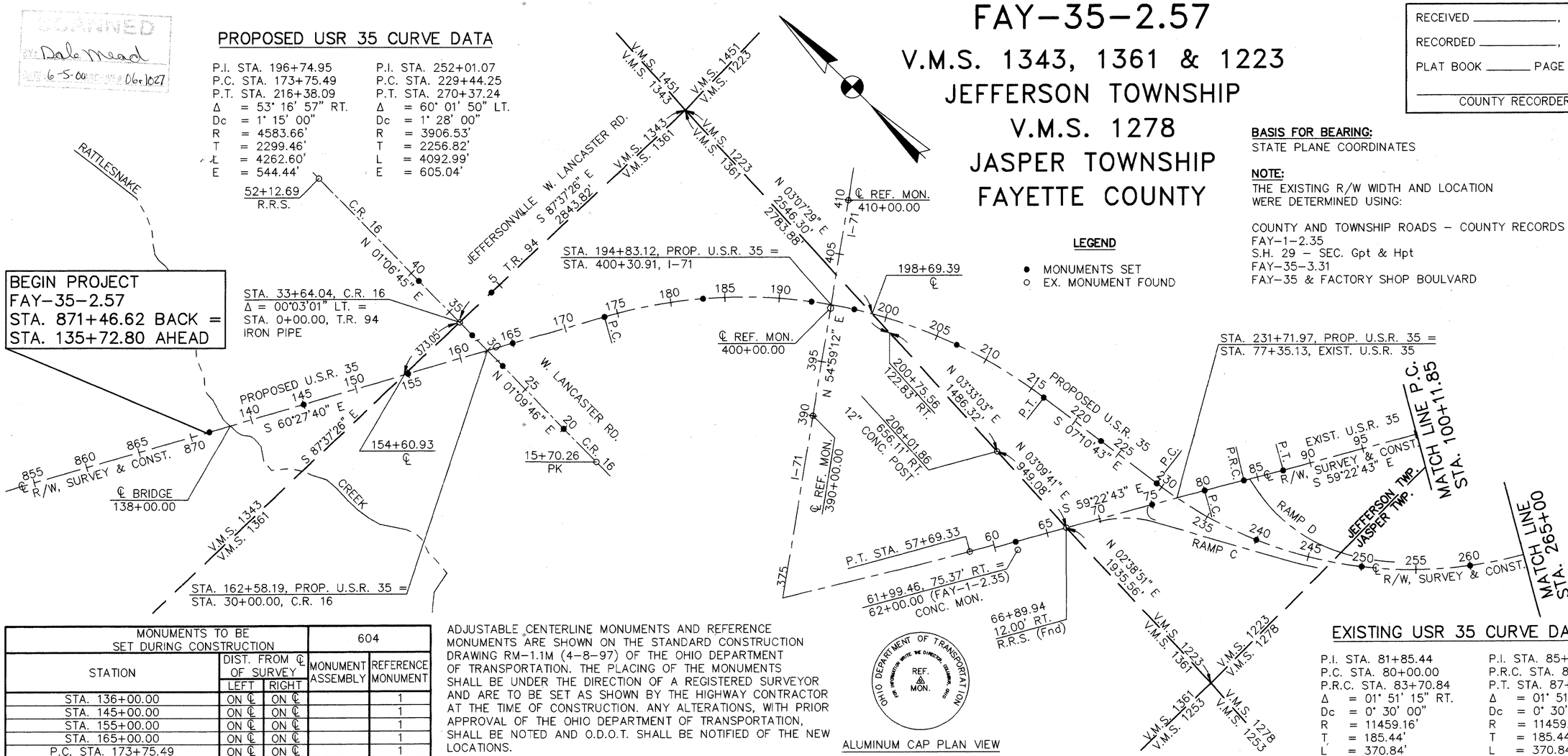
NOTE:
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS
FAY-1-2.35
S.H. 29 – SEC. Gpt & Hpt
FAY-35-3.31
FAY-35 & FACTORY SHOP BOULVARD

LEGEND
- ● MONUMENTS SET
- ○ EX. MONUMENT FOUND

PROPOSED USR 35 CURVE DATA

P.I. STA. 196+74.95	P.I. STA. 252+01.07
P.C. STA. 173+75.49	P.C. STA. 229+44.25
P.T. STA. 216+38.09	P.T. STA. 270+37.24
Δ = 53° 16' 57" RT.	Δ = 60° 01' 50" LT.
Dc = 1° 15' 00"	Dc = 1° 28' 00"
R = 4583.66'	R = 3906.53'
T = 2299.46'	T = 2256.82'
L = 4262.60'	L = 4092.99'
E = 544.44'	E = 605.04'

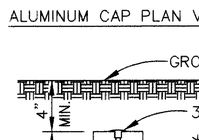


EXISTING USR 35 CURVE DATA

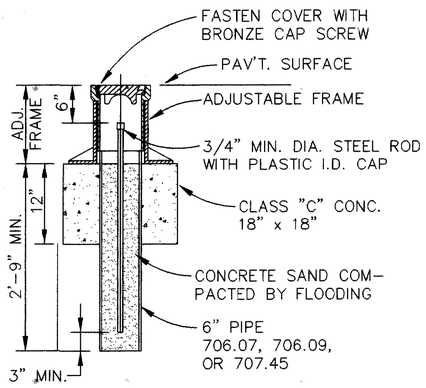
P.I. STA. 81+85.44	P.I. STA. 85+56.28
P.C. STA. 80+00.00	P.R.C. STA. 83+70.84
P.R.C. STA. 83+70.84	P.T. STA. 87+41.68
Δ = 01° 51' 15" RT.	Δ = 01° 51' 15" LT.
Dc = 0° 30' 00"	Dc = 0° 30' 00"
R = 11459.16'	R = 11459.16'
T = 185.44'	T = 185.44'
L = 370.84'	L = 370.84'
E = 6.00'	E = 6.00'

MONUMENTS TO BE SET DURING CONSTRUCTION	DIST. FROM ℄ OF SURVEY		604	
STATION	LEFT	RIGHT	MONUMENT ASSEMBLY	REFERENCE MONUMENT
STA. 136+00.00	ON ℄	ON ℄		1
STA. 145+00.00	ON ℄	ON ℄		1
STA. 155+00.00	ON ℄	ON ℄		1
STA. 165+00.00	ON ℄	ON ℄		1
P.C. STA. 173+75.49	ON ℄	ON ℄		1
STA. 183+00.00	ON ℄	ON ℄		1
STA. 193+00.00	ON ℄	ON ℄		1
STA. 197+00.00	ON ℄	ON ℄		1
STA. 207+00.00	ON ℄	ON ℄		1
P.T. STA. 216+38.09	ON ℄	ON ℄		1
STA. 223+00.00	ON ℄	ON ℄		1
P.C. STA. 229+44.25	ON ℄	ON ℄		1
STA. 240+00.00	ON ℄	ON ℄		1
STA. 250+00.00	ON ℄	ON ℄		1
STA. 260+00.00	ON ℄	ON ℄		1
STA. 20+00.00 C.R. 16	ON ℄	ON ℄	1	
STA. 28+00.00 C.R. 16	ON ℄	ON ℄	1	
STA. 32+00.00 C.R. 16	ON ℄	ON ℄	1	
STA. 33+64.04 C.R. 16	ON ℄	ON ℄	1	
STA. 39+00.00 C.R. 16	ON ℄	ON ℄	1	
STA. 4+00.00 T.R. 94	ON ℄	ON ℄	1	
STA. 62+00.00 EX. U.S.R. 35	ON ℄	ON ℄	1	
STA. 75+00.00 EX. U.S.R. 35	ON ℄	ON ℄	1	
P.C. STA. 80+00.00 EX. U.S.R. 35	ON ℄	ON ℄	1	
P.R.C. STA. 83+70.84 EX. U.S.R. 35	ON ℄	ON ℄	1	
P.T. STA. 87+41.68 EX. U.S.R. 35	ON ℄	ON ℄	1	
TOTAL TO SHEET 42			11	15

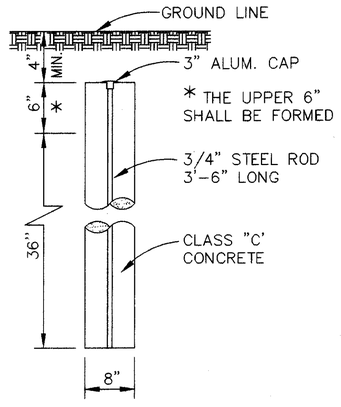
ADJUSTABLE CENTERLINE MONUMENTS AND REFERENCE MONUMENTS ARE SHOWN ON THE STANDARD CONSTRUCTION DRAWING RM-1.1M (4-8-97) OF THE OHIO DEPARTMENT OF TRANSPORTATION. THE PLACING OF THE MONUMENTS SHALL BE UNDER THE DIRECTION OF A REGISTERED SURVEYOR AND ARE TO BE SET AS SHOWN BY THE HIGHWAY CONTRACTOR AT THE TIME OF CONSTRUCTION. ANY ALTERATIONS, WITH PRIOR APPROVAL OF THE OHIO DEPARTMENT OF TRANSPORTATION, SHALL BE NOTED AND O.D.O.T. SHALL BE NOTIFIED OF THE NEW LOCATIONS.



ALUMINUM CAP PLAN VIEW



ADJUSTABLE CENTERLINE MONUMENT DETAIL



REFERENCE MONUMENT DETAIL

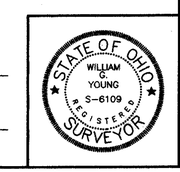
THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE REVISED CODE OF OHIO.

I HEREBY CERTIFY THAT THIS PLAT IS A TRUE DELINEATION OF A SURVEY MADE FOR THE OHIO DEPARTMENT OF TRANSPORTATION IN 1995 BY ERIKSSON ENGINEERING LIMITED.

THE ESTABLISHMENT OF THE PROPERTY LINES AND EXISTING RIGHT OF WAY LINES SHOWN ON THIS PLAN AS OF THIS DATE WAS PERFORMED BY ME OR UNDER MY DIRECT SUPERVISION.

BY William G. Young
SURVEYOR'S NAME

SURVEYOR NO. 6109 DATE 1-8-99



CENTERLINE SURVEY PLAT
STA. 135+72.80 TO STA. 265+00

FAY-35-2.57

1/40

483/522

Q:\3590\RWCLP01

FAY-35-2.57
V.M.S. 1278 & 1342
JASPER TOWNSHIP
FAYETTE COUNTY

RECEIVED _____, 19____
RECORDED _____, 19____
PLAT BOOK _____ PAGE _____
_____
COUNTY RECORDER

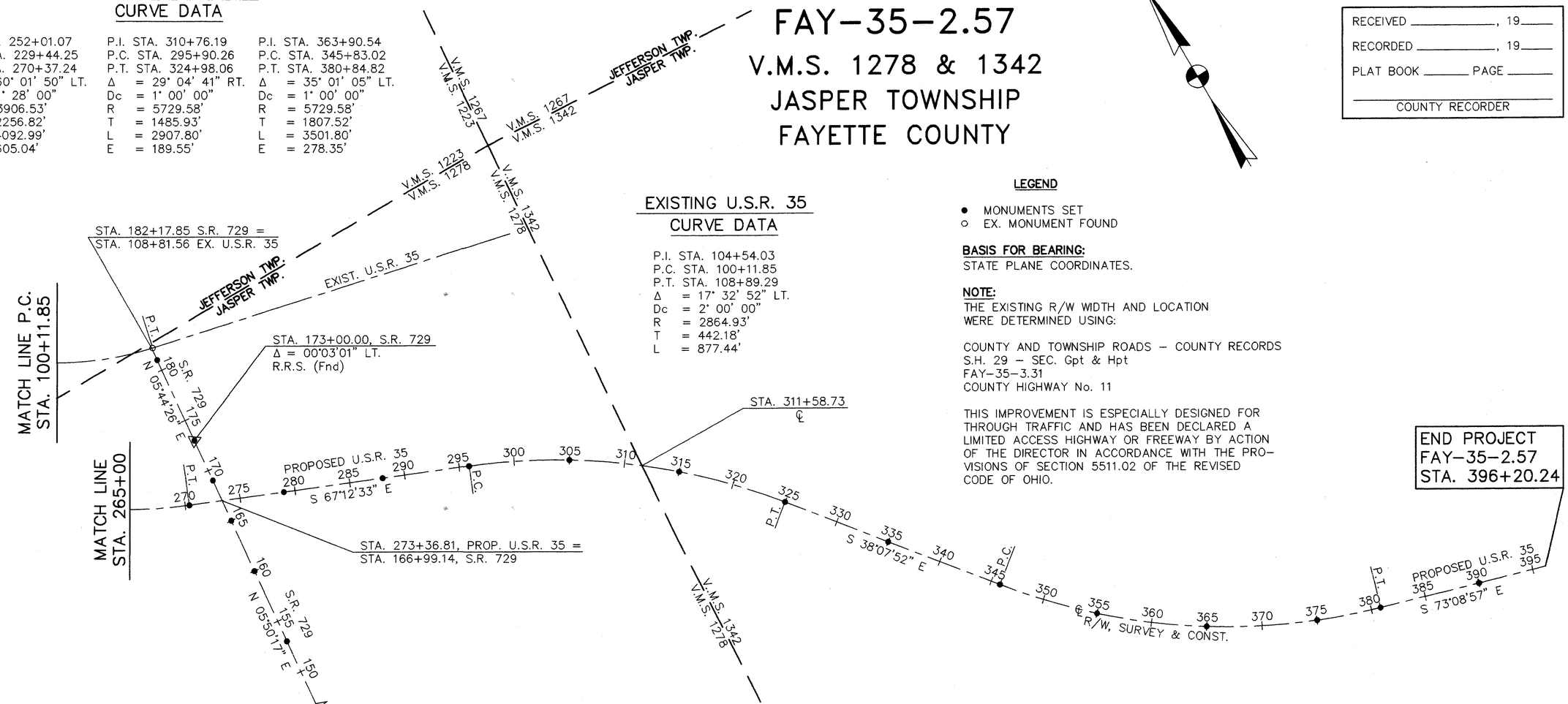
P I D NO. 9078

PROPOSED U.S.R. 35 CURVE DATA

P.I. STA. 252+01.07	P.I. STA. 310+76.19	P.I. STA. 363+90.54
P.C. STA. 229+44.25	P.C. STA. 295+90.26	P.C. STA. 345+83.02
P.T. STA. 270+37.24	P.T. STA. 324+98.06	P.T. STA. 380+84.82
Δ = 60° 01' 50" LT.	Δ = 29° 04' 41" RT.	Δ = 35° 01' 05" LT.
Dc = 1° 28' 00"	Dc = 1° 00' 00"	Dc = 1° 00' 00"
R = 3906.53'	R = 5729.58'	R = 5729.58'
T = 2256.82'	T = 1485.93'	T = 1807.52'
L = 4092.99'	L = 2907.80'	L = 3501.80'
E = 605.04'	E = 189.55'	E = 278.35'

EXISTING U.S.R. 35 CURVE DATA

P.I. STA. 104+54.03
P.C. STA. 100+11.85
P.T. STA. 108+89.29
Δ = 17° 32' 52" LT.
Dc = 2° 00' 00"
R = 2864.93'
T = 442.18'
L = 877.44'



LEGEND

- ● MONUMENTS SET
- ○ EX. MONUMENT FOUND

BASIS FOR BEARING:
STATE PLANE COORDINATES.

NOTE:
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

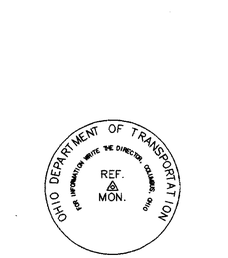
COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS
S.H. 29 – SEC. Gpt & Hpt
FAY-35-3.31
COUNTY HIGHWAY No. 11

THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE REVISED CODE OF OHIO.

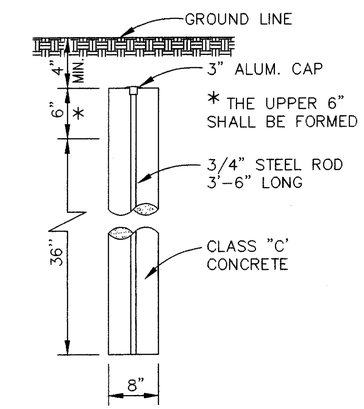
END PROJECT
FAY-35-2.57
STA. 396+20.24

CENTERLINE SURVEY PLAT
STA. 265+00 TO STA. 396+20.24

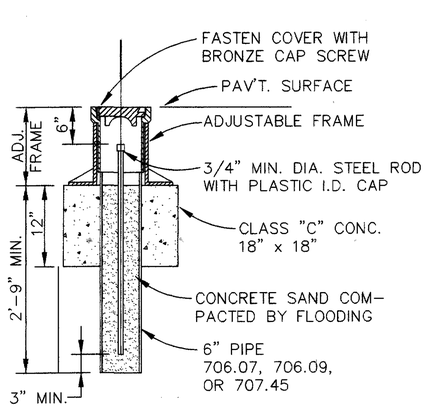
FAY-35-2.57



ALUMINUM CAP PLAN VIEW



REFERENCE MONUMENT DETAIL



ADJUSTABLE CENTERLINE MONUMENT DETAIL

MONUMENTS TO BE SET DURING CONSTRUCTION			604	
STATION	DIST. FROM ℄ OF SURVEY LEFT	RIGHT	MONUMENT ASSEMBLY	REFERENCE MONUMENT
P.T. STA. 270+37.24	ON ℄	ON ℄		1
STA. 279+00.00	ON ℄	ON ℄		1
STA. 288+00.00	ON ℄	ON ℄		1
P.C. STA. 295+90.26	ON ℄	ON ℄		1
SRA. 305+00.00	ON ℄	ON ℄		1
STA. 315+00.00	ON ℄	ON ℄		1
P.T. STA. 324+98.06	ON ℄	ON ℄		1
STA. 335+00.00	ON ℄	ON ℄		1
P.C. STA. 345+83.02	ON ℄	ON ℄		1
STA. 355+00.00	ON ℄	ON ℄		1
STA. 365+00.00	ON ℄	ON ℄		1
STA. 375+00.00	ON ℄	ON ℄		1
P.T. STA. 380+84.82	ON ℄	ON ℄		1
STA. 390+00.00	ON ℄	ON ℄		1
STA. 153+00.00 S.R. 729	ON ℄	ON ℄	1	
STA. 160+00.00 S.R. 729	ON ℄	ON ℄	1	
STA. 165+00.00 S.R. 729	ON ℄	ON ℄	1	
STA. 169+00.00 S.R. 729	ON ℄	ON ℄	1	
STA. 173+00.00 S.R. 729	ON ℄	ON ℄	1	
STA. 181+00.00 S.R. 729	ON ℄	ON ℄	1	
TOTAL TO SHEET 42			6	14

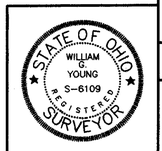
ADJUSTABLE CENTERLINE MONUMENTS AND REFERENCE MONUMENTS ARE SHOWN ON THE STANDARD CONSTRUCTION DRAWING RM-1.1M (4-8-97) OF THE OHIO DEPARTMENT OF TRANSPORTATION. THE PLACING OF THE MONUMENTS SHALL BE UNDER THE DIRECTION OF A REGISTERED SURVEYOR AND ARE TO BE SET AS SHOWN BY THE HIGHWAY CONTRACTOR AT THE TIME OF CONSTRUCTION. ANY ALTERATIONS, WITH PRIOR APPROVAL OF THE OHIO DEPARTMENT OF TRANSPORTATION, SHALL BE NOTED AND O.D.O.T. SHALL BE NOTIFIED OF THE NEW LOCATIONS.

I HEREBY CERTIFY THAT THIS PLAT IS A TRUE DELINEATION OF A SURVEY MADE FOR THE OHIO DEPARTMENT OF TRANSPORTATION IN 1995 BY ERIKSSON ENGINEERING LIMITED.

THE ESTABLISHMENT OF THE PROPERTY LINES AND EXISTING RIGHT OF WAY LINES SHOWN ON THIS PLAN AS OF THIS DATE WAS PERFORMED BY ME OR UNDER MY DIRECT SUPERVISION.

BY William G. Young
SURVEYOR'S NAME

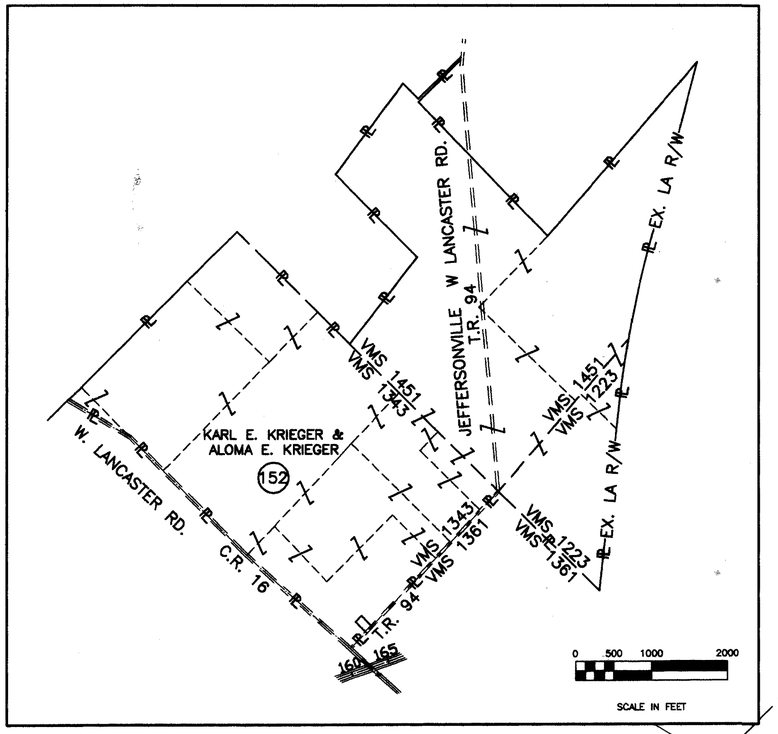
SURVEYOR NO. 6109 DATE 1-8-99

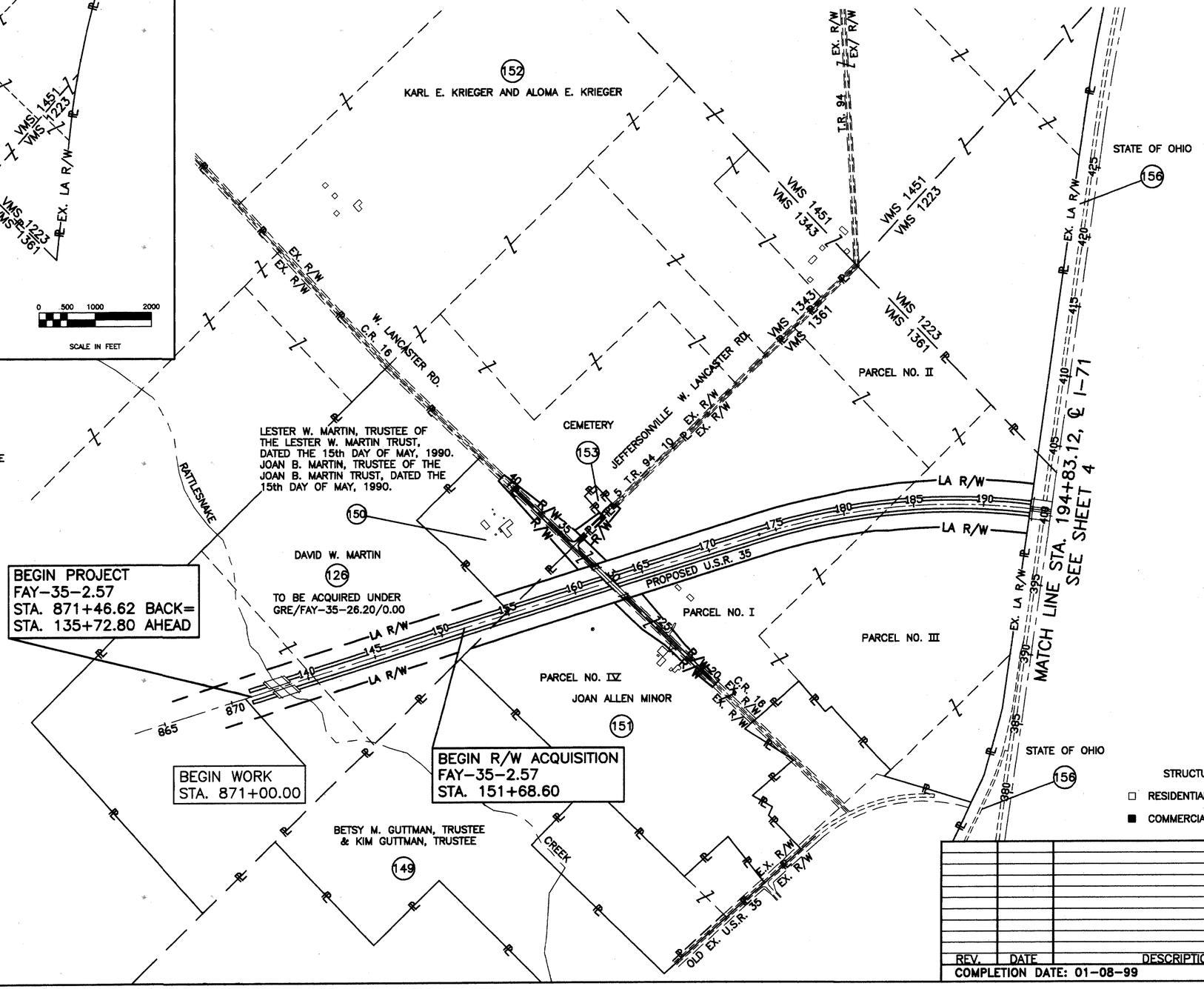


2 / 40
484 / 522

Q:\3590\RWCLP02

FAY–35–2.57
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343 & 1361





UTILITY OWNERS

DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937)331–4498

AT&T COMMUNICATIONS
3972 BROWN PARK DRIVE
SUITE D
HILLIARD, OHOI 43026
(614)876–9366

AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223–8535

SANITARY SEWERS
FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(790)335–1541

FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740)775–4288

CNG TRANSMISSION CORP.
PO BOX 2450
CLARKSBURG, WV 26302–2450
(304)623–8454

DUKE ENERGY
5444 WESTHEIMER
WT 610
PO BOX 1642
HOUSTON, TEXAS 77251–1642
(713)989–2232

TEXAS EASTERN PRODUCTS PIPELINE COMPANY
P.O. BOX 426
SEYMOUR, INDIANA 47274–0426
(812)522–2569

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.

SCALE IN FEET 0 200 400 800

P I D NO. 9078

PROPERTY MAP
STA. 130+00 TO STA. 194+83.12

FAY-35-2.57

REV.	DATE	DESCRIPTION

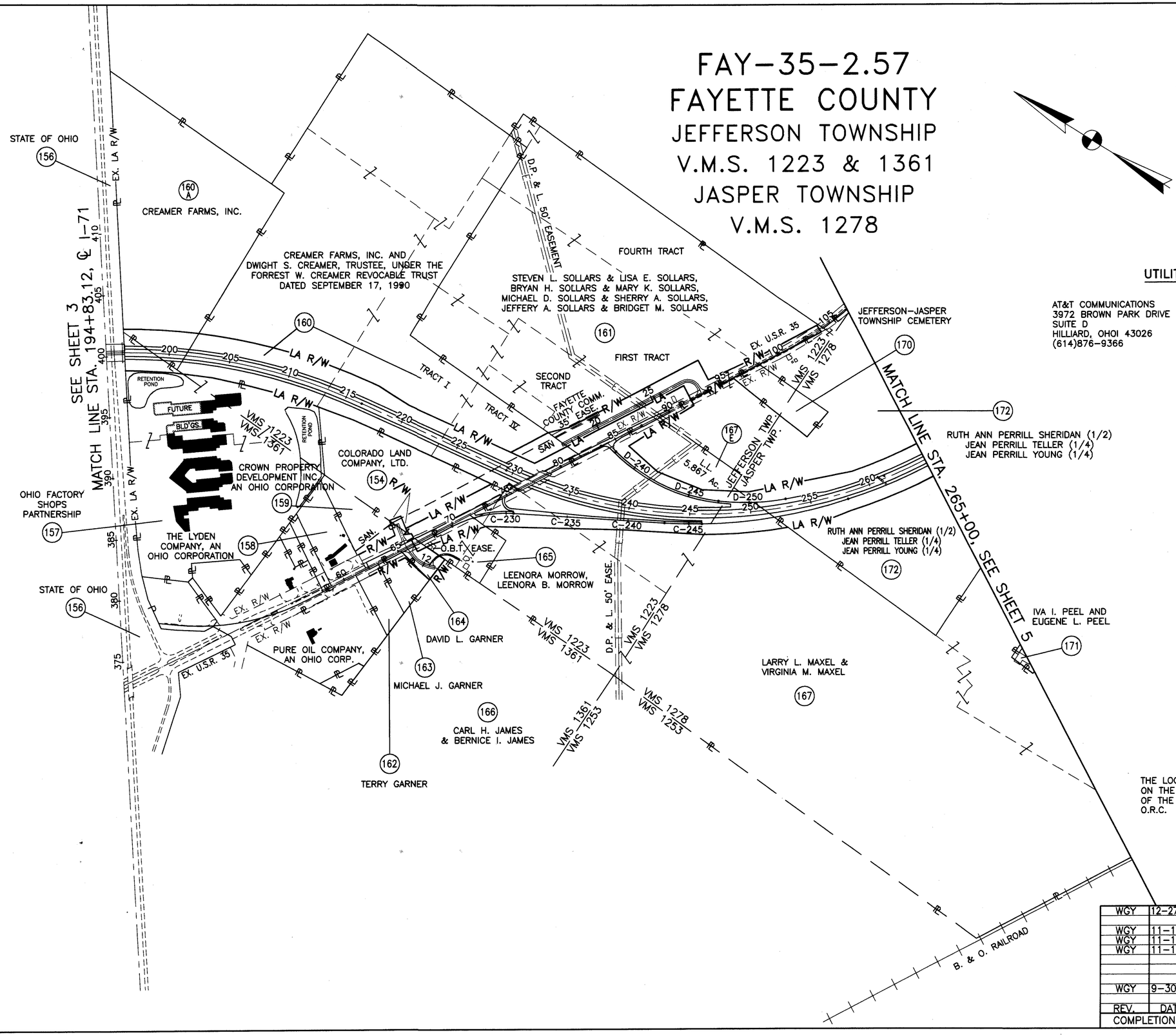
COMPLETION DATE: 01–08–99

3 / 40

485 / 522

Q:\3590\3590PR01

FAY-35-2.57
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223 & 1361
JASPER TOWNSHIP
V.M.S. 1278



UTILITY OWNERS

AT&T COMMUNICATIONS
3972 BROWN PARK DRIVE
SUITE D
HILLIARD, OHOI 43026
(614)876-9366

DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937)331-4498

AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223-8535

SANITARY SEWERS
FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(790)335-1541

FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740)775-4288

CNG TRANSMISSION CORP.
PO BOX 2450
CLARKSBURG, WV 26302-2450
(304)623-8454

DUKE ENERGY
5444 WESTHEIMER
WT 610
PO BOX 1642
HOUSTON, TEXAS 77251-1642
(713)989-2232

TEXAS EASTERN PRODUCTS PIPELINE COMPANY
P.O. BOX 426
SEYMOUR, INDIANA 47274-0426
(812)522-2569

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.

STRUCTURE KEY

□ RESIDENTIAL BUILDINGS

■ COMMERCIAL BUILDINGS

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 160 A
WGY	11-1-99	ADDED PARCEL 154
WGY	11-1-99	ADDED PARCEL 167 E
WGY	11-1-99	REVISED PARCEL 160
WGY	9-30-99	REVISED PARCELS 162, 163, AND 164

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0, 200, 400, 800

P I D NO. 9078

PROPERTY MAP
STA. 194+83.12 TO STA. 265+00

FAY-35-2.57

4 / 40

486 / 522

Q:\3590\3590PR02

FAY-35-2.57
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223 & 1267
JASPER TOWNSHIP
V.M.S. 1278 & 1342

UTILITY OWNERS

DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937)331-4498

AT&T COMMUNICATIONS
3972 BROWN PARK DRIVE
SUITE D
HILLIARD, OHOI 43026
(614)876-9366

AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223-8535

SANITARY SEWERS
FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(790)335-1541

FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740)775-4288

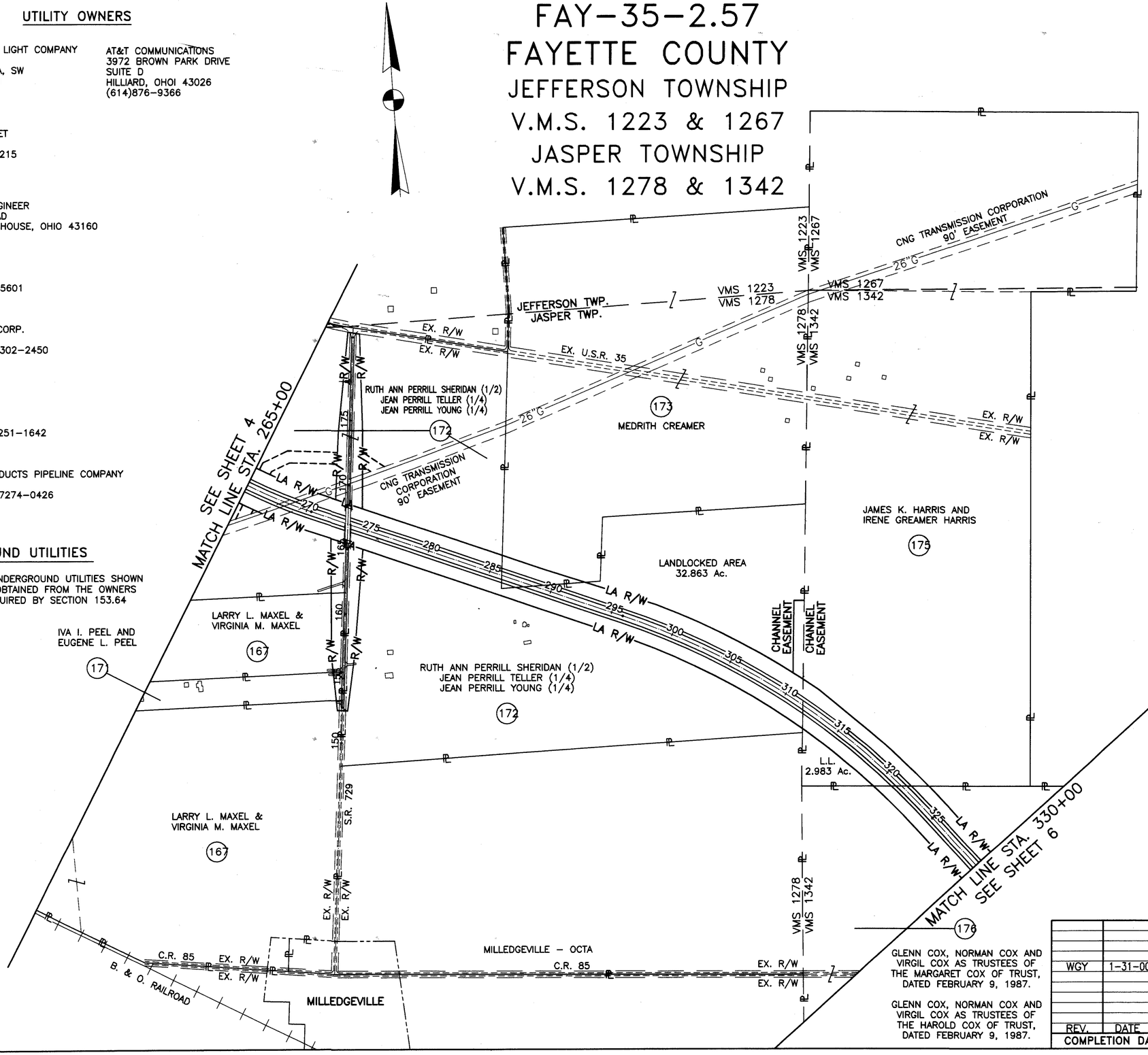
CNG TRANSMISSION CORP.
PO BOX 2450
CLARKSBURG, WV 26302-2450
(304)623-8454

DUKE ENERGY
5444 WESTHEIMER
WT 610
PO BOX 1642
HOUSTON, TEXAS 77251-1642
(713)989-2232

TEXAS EASTERN PRODUCTS PIPELINE COMPANY
P.O. BOX 426
SEYMOUR, INDIANA 47274-0426
(812)522-2569

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.



GLENN COX, NORMAN COX AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX OF TRUST, DATED FEBRUARY 9, 1987.

GLENN COX, NORMAN COX AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX OF TRUST, DATED FEBRUARY 9, 1987.

STRUCTURE KEY

- □ RESIDENTIAL BUILDINGS
- ■ COMMERCIAL BUILDINGS

REV.	DATE	DESCRIPTION
WGY	1-31-00	CHANGED NAME TO CNG TRANSMISSION CORP. ADDED UTILITY EASEMENT

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0 200 400 800

P I D NO. 9078

PROPERTY MAP
STA. 265+00 TO STA. 330+00

FAY-35-2.57

5 / 40

487 / 522

Q:\3590\3590PR03

FAY-35-2.57
FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

SEE SHEET 5
MATCH LINE STA. 330+00

GLENN COX, NORMAN COX AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX OF TRUST, DATED FEBRUARY 9, 1987.

GLENN COX, NORMAN COX AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX OF TRUST, DATED FEBRUARY 9, 1987.

176

LA R/W
335 340 345 350 355 360 365 370 375 380 385 390 395
LA R/W
LA R/W
LA R/W

780 C.R. 85
785 C.R. 85
7 Ex. R/W
Ex. R/W
Ex. R/W
Ex. R/W

T.R. 867

LANDLOCKED AREA 9.123 Ac.

THEODORE D. CRAIG AND DAVID W. CRAIG
178

CHANNEL EASEMENT

L.L. 2.837 Ac.

McDONALDS FARMS, INC.
179
TO BE ACQUIRED UNDER FAY-35-7.51

TEXAS EASTERN TRANSMISSION CORP. (NO R/W WIDTH)

FONDA GAY FICHTHORN
177

B 59° 28' 55" E 24.95' 20" G

VMS 1379
VMS 1342
Ex. R/W
C.R. 74
HARMONY RD.

END WORK
STA. 396+44.86
(BACK STATION)

END R/W ACQUISITION
FAY-35-2.57
STA. 399+00.35
(BACK STATION)

END PROJECT
FAY-35-2.57
STA. 396+20.24 BACK =
STA. 380+00.00 AHEAD

B. & O. RAILROAD
B. & O. RAILROAD

VMS 1342
VMS 1278

VMS 1379
VMS 1342
C.R. 74
Ex. R/W
Ex. R/W

UTILITY OWNERS

AT&T COMMUNICATIONS
3972 BROWN PARK DRIVE
SUITE D
HILLIARD, OHOI 43026
(614)876-9366

DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937)331-4498

AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223-8535

SANITARY SEWERS
FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(790)335-1541

FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740)775-4288

CNG TRANSMISSION CORP.
PO BOX 2450
CLARKSBURG, WV 26302-2450
(304)623-8454

DUKE ENERGY
5444 WESTHEIMER
WT 610
PO BOX 1642
HOUSTON, TEXAS 77251-1642
(713)989-2232

TEXAS EASTERN PRODUCTS PIPELINE COMPANY
P.O. BOX 426
SEYMOUR, INDIANA 47274-0426
(812)522-2569

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.

STRUCTURE KEY
- □ RESIDENTIAL BUILDINGS
- ■ COMMERCIAL BUILDINGS

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 200 400 800

P I D NO. 9078

PROPERTY MAP
STA. 330+00 TO STA. 405+00

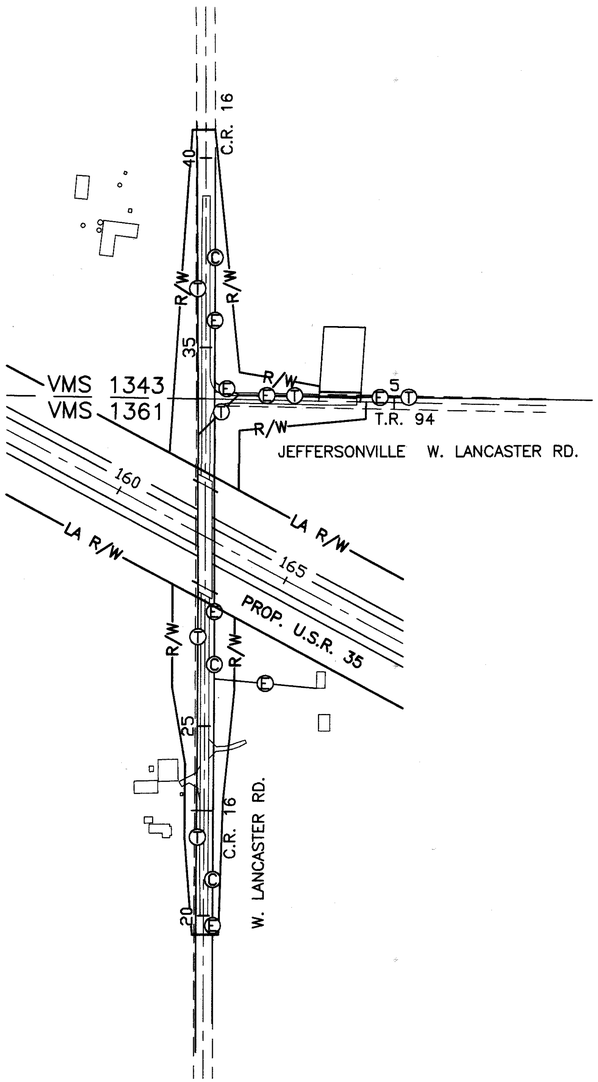
FAY-35-2.57

6 / 40

488
522

Q:\3590\3590PR04

FAY-35-2.57
V.M.S. 1343 & 1361
JEFFERSON TOWNSHIP
V.M.S. 1278
JASPER TOWNSHIP
FAYETTE COUNTY



C.R. 16 & T.R. 94

UTILITY OWNERS

(E) DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937) 331-4498

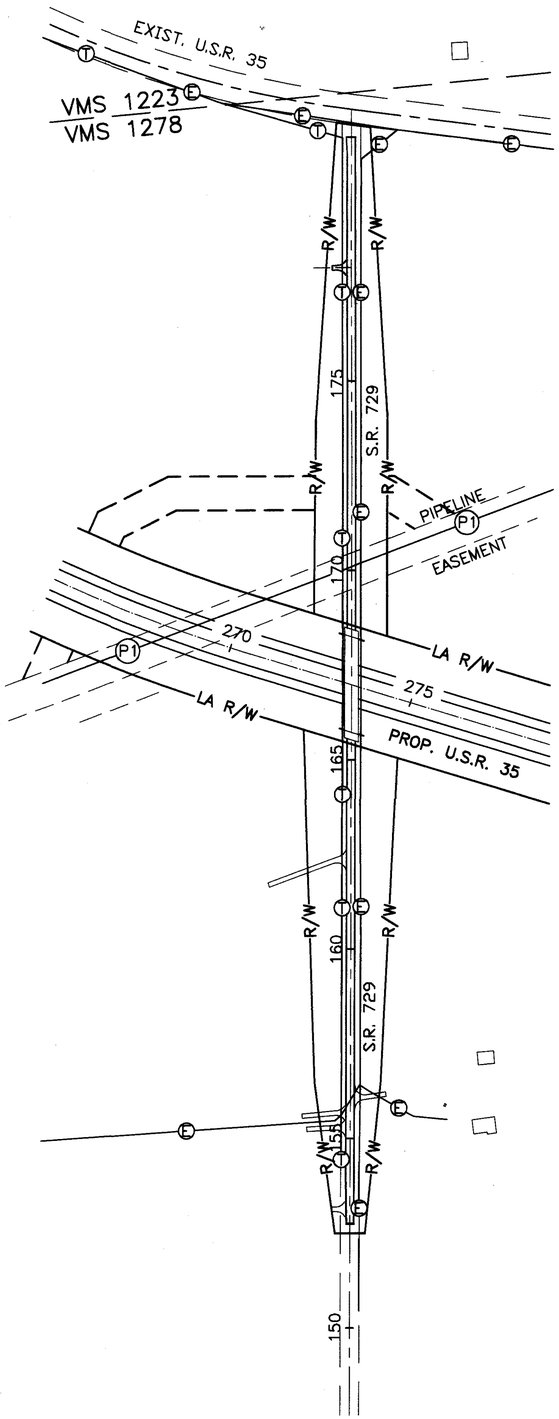
(T) AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223-8535

(C) FRONTIER VISION
363 MARIETTA PIKE
CHILLICOTHE, OHIO 45601
(740)775-4288

(P1) CNG TRANSMISSION CORP.
PO BOX 2450
CLARKSBURG, WV 26302-2450
(304)623-8454

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.



S.R. 729

P I D NO. 9078

UTILITY PLAN

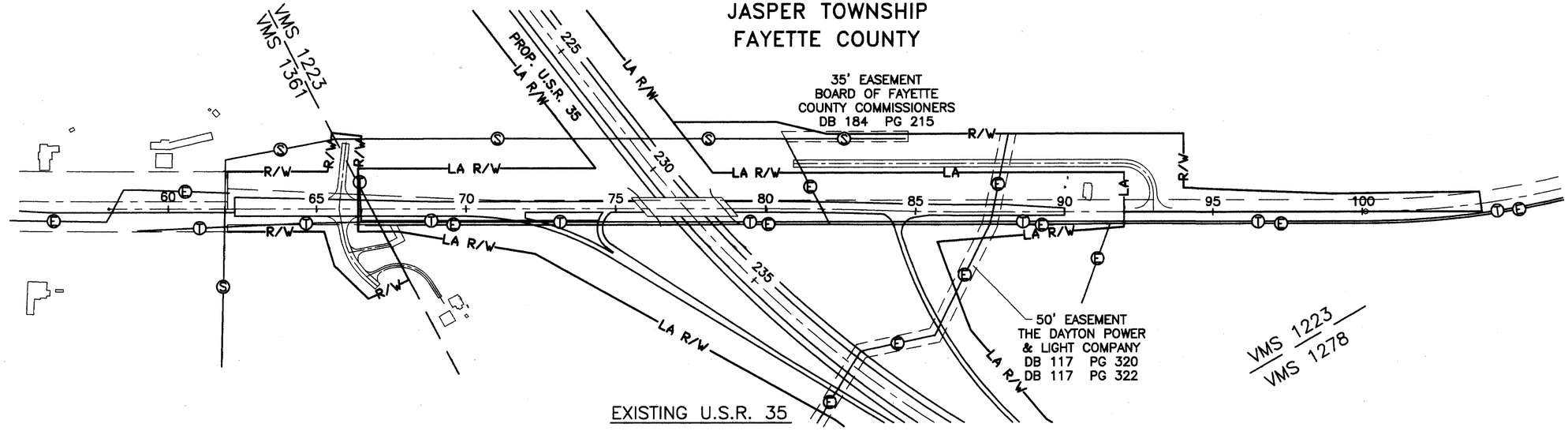
FAY-35-2.57

7/40

489/522

Q:\3590\RWUTL01

FAY-35-2.57
V.M.S. 1223 & 1361
JEFFERSON TOWNSHIP
V.M.S. 1242
JASPER TOWNSHIP
FAYETTE COUNTY



UTILITY OWNERS

(E) DAYTON POWER AND LIGHT COMPANY
P.O. BOX 1247
COURT HOUSE PLAZA, SW
DAYTON, OHIO 45401
(937)331-4498

(T) AMERITECH
150 EAST GAY STREET
ROOM 6C
COLUMBUS, OHIO 43215
(614)223-8535

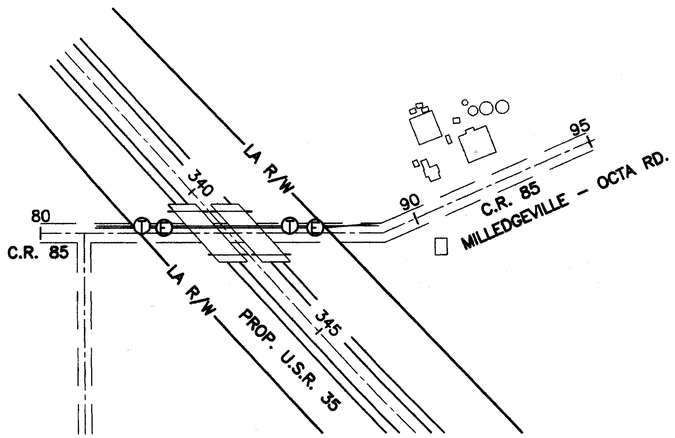
(S) FAYETTE COUNTY ENGINEER
1600 ROBINSON ROAD
WASHINGTON COURT HOUSE, OHIO 43160
(790)335-1541

(P2) DUKE ENERGY
5444 WESTHEIMER
WT 614
PO BOX 1642
HOUSTON, TEXAS 77251-1642
(713)989-2232

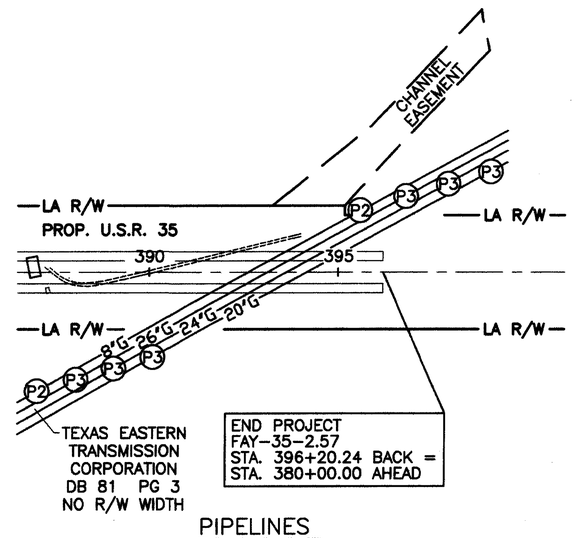
(P3) TEXAS EASTERN PRODUCTS PIPELINE COMPANY
P.O. BOX 426
SEYMOUR, INDIANA 47274-0426
(812)522-2569

UNDERGROUND UTILITIES

THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN ON THE PLANS ARE AS OBTAINED FROM THE OWNERS OF THE UTILITIES AS REQUIRED BY SECTION 153.64 O.R.C.



C.R. 85



PIPELINES

N

SCALE IN FEET
0 100 200 400

P I D NO. 9078

UTILITY MAP

FAY-35-2.57

8 / 40

490 / 522

Q:\3590\RWUTL02

TOTAL NUMBERS OF
21 OWNERSHIPS
1 TOTAL TAKES
3 OWNERSHIPS WITH STRUCTURES INVOLVED
____ OWNERSHIPS WITH "P" ITEMS

GRANTEE:
ALL RIGHT OF WAY ACQUIRED IN THE NAME
OF STATE OF OHIO UNLESS OTHERWISE SHOWN.

P I D NO. 9078

ALL AREAS IN ACRES

NET RESIDUE = RECORD AREA – TOTAL P.R.O. – NET TAKE

PARCEL NO.	OWNER	SHEET NO.	OWNERS RECORD BOOK	OWNERS RECORD PAGE	AUDITOR'S PARCEL	RECORD AREA	TOTAL P.R.O.	GROSS TAKE	P.R.O. IN TAKE	NET TAKE	STRUC-TURE	NET RESIDUE LEFT	NET RESIDUE RIGHT	TYPE FUND	REMARKS AND PERSONALTY	AS ACQUIRED BOOK	AS ACQUIRED PAGE
149	BETSY M. GUTTMAN, TRUSTEE & KIM GUTTMAN, TRUSTEE													STATE	NO R/W TAKE REQUIRED		
150WL	LESTER W. MARTIN, TRUSTEE OF THE LESTER W. MARTIN	14,15	176	917	060-011-0-00-001-01	13.587	0.529	0.458	0	0.458	NO						
	TRUST, DATED THE 15th DAY OF MAY, 1990.		176	923													
150WV	JOAN B. MARTIN, TRUSTEE OF THE JOAN B. MARTIN	34						0.978	0.408	0.570	NO				+		
	TRUST, DATED THE 15th DAY OF MAY, 1990.																
	GRAND TOTAL					13.587	0.529	1.436	0.408	1.028	NO	12.030					
151WL	JOAN ALLEN MINOR	14,15	151	351	060-015-0-00-021-00	225.52	3.469	27.796	0.344	27.452	NO				13.421 ACRES OUT OF NO. 060-015-0-00-021-00		
		16,17			060-015-0-00-023-00										14.375 ACRES OUT OF NO. 060-015-0-00-023-00		
151WV		33						2.453	1.010	1.443	NO				+ 1.262 ACRES OUT OF NO. 060-015-0-00-021-00		
															1.191 ACRES OUT OF NO. 060-015-0-00-023-00		
151WV1		34						0.800	0.249	0.551	NO				+ 0.352 ACRE OUT OF NO. 060-015-0-00-021-00		
															0.448 ACRE OUT OF NO. 060-015-0-00-023-00		
151WV2		34						0.488	0.138	0.350	NO				* 0.488 ACRE OUT OF NO. 060-015-0-00-023-00		
	GRAND TOTAL					225.52	3.469	31.537	1.741	29.796	NO	65.348	126.907				
151T		33						0.040	–	0.040					TO CONSTRUCT A RESIDENTIAL DRIVE		
151T–1		33						0.097	–	0.097					TO CONSTRUCT A FIELD DRIVE		
152WV	KARL E. KRIEGER & ALOMA E. KRIEGER	34	127	195	060-011-0-00-013-00	673.69	7.746	0.993	0.435	0.558	NO				+		
152WV1		34						0.274	0.087	0.187	NO				*		
	GRAND TOTAL					673.69	7.746	1.267	0.522	0.745	NO	665.199	–				
153	CEMETERY	34													NO R/W TAKE REQUIRED		
154WL	COLORADO LAND COMPANY, LTD.	20,35	50	889	060-016-0-00-004-01	18.355	0.878	2.811	0.879	1.932	NO						
154WD		35	50	894				0.178		0.178	NO						
	GRAND TOTAL						0.878	2.989	0.879	2.110		15.367					
154T								1.440		1.440	NO				TO CONSTRUCT AND REMOVE TEMPORARY		
155	NOT USED														RUNAROUND & CONSTRUCT A SAN. MANHOLE		
156	STATE OF OHIO														NO R/W TAKE REQUIRED		
157WL	OHIO FACTORY SHOPS PARTNERSHIP	17,18	173	997	060-015-0-00-027-00	43.701	–	1.699	–	1.699	NO	–	49.608		1.656 ACRES OUT OF NO. 060-015-0-00-027-00		
			182	778	060-016-0-00-006-02	7.606									0.043 ACRE OUT OF NO. 060-016-0-00-006-02		
158	THE LYDEN COMPANY, AN OHIO		182	073											NO R/W TAKE REQUIRED		
	CORPORATION																
159WL	CROWN PROPERTY DEVELOPMENT INC.,	35	168	93	060-015-0-00-037-00	5.299	0.757	0.055	0.046	0.009	NO						
159WD	AN OHIO CORPORATION	35						1.496	0.711	0.785	NO						
	GRAND TOTAL					5.299	0.757	1.551	0.757	0.794	NO	3.748	–				
159T		35						0.272	–	0.272				STATE	TO CONSTRUCT & REMOVE TEMP. RUN AROUND		

NOTE: UNDER NO CIRCUMSTANCES ARE TEMPORARY EASEMENTS ACQUIRED FOR THE PURPOSE OF STRUCTURAL REMOVAL TO BE USED FOR STORAGE OF MATERIAL OR EQUIPMENT BY THE CONTRACTOR. UPON COMPLETION OF THE WORK REQUIRED FOR SUCH REMOVAL AND SUBSEQUENT RECLAMATION, THE EASEMENT SHALL BE VACATED IMMEDIATELY.

NOTE: ALL TEMPORARY PARCELS TO BE OF 18 MONTHS DURATION.

* RIGHT OF WAY ACQUIRED IN THE NAME OF THE COUNTY OF FAYETTE.

* RIGHT OF WAY ACQUIRED IN THE NAME OF THE TOWNSHIP OF JEFFERSON.

REV.	DATE	DESCRIPTION
WGY	11–1–99	ADDED PARCEL 154

COMPLETION DATE: 01–08–99

SUMMARY OF ADDITIONAL RIGHT OF WAY PARCELS 149 – 159

FAY–35–2.57

Q:\3590\RWSUM01

TOTAL NUMBERS OF

_____ OWNERSHIPS
_____ TOTAL TAKES
_____ OWNERSHIPS WITH STRUCTURES INVOLVED
_____ OWNERSHIPS WITH "P" ITEMS

GRANTEE:
ALL RIGHT OF WAY ACQUIRED IN THE NAME OF STATE OF OHIO UNLESS OTHERWISE SHOWN.

P I D NO. 9078

ALL AREAS IN ACRES

NET RESIDUE = RECORD AREA – TOTAL P.R.O. – NET TAKE

PARCEL NO.	OWNER	SHEET NO.	OWNERS RECORD BOOK	OWNERS RECORD PAGE	AUDITOR'S PARCEL	RECORD AREA	TOTAL P.R.O.	GROSS TAKE	P.R.O. IN TAKE	NET TAKE	STRUCTURE	NET RESIDUE LEFT	NET RESIDUE RIGHT	TYPE FUND	REMARKS AND PERSONALTY	AS ACQUIRED BOOK	AS ACQUIRED PAGE
160WL	CREAMER FARMS, INC. AND DWIGHT S.	18	145	357	060-016-0-00-004-00	127.724	0.602	23.610	0.602	23.008	NO			STATE	7.538 ACRES OUT OF NO. 060-016-0-00-004-00		
	CREAMER, TRUSTEE, UNDER THE FORREST	19,20	179	693	060-016-0-00-006-00										9.520 ACRES OUT OF NO. 060-016-0-00-006-00		
	W. CREAMER REVOCABLE TRUST DATED	35			060-015-0-00-037-00										6.552 ACRES OUT OF NO. 060-015-0-00-037-00		
	SEPTEMBER 17, 1990.																
160WD-1		20						1.261	–	1.261	NO				1.261 ACRES OUT OF NO. 060-015-0-00-037-00		
	GRAND TOTAL					127.724	0.602	24.871	0.602	24.269	NO	102.853					
160A-WL	CREAMER FARMS, INC.	17,18	145	355	060-016-0-00-006-01	43.781	–	2.382	–	2.382	NO	41.399					
161WL	STEVEN L.SOLLARS & LISA E. SOLLARS,	20,36	182 / 182	347 / 509	060-016-0-00-007-01	115.510	2.780	3.550	1.201	2.349	YES				2.139 ACRES OUT OF NO. 060-016-0-00-007-01		
	BRYAN H. SOLLARS & MARY K. SOLLARS,				060-016-0-00-008-00										1.411 ACRES OUT OF NO. 060-016-0-00-008-00		
161WD	MICHAEL D. SOLLARS & SHERRY A. SOLLARS,	20,36			060-016-0-00-009-00			6.103	1.178	4.925	NO				3.082 ACRES OUT OF NO. 060-016-0-00-008-00		
	JEFFERY A. SOLLARS & BRIDGET M. SOLLARS.	37													2.173 ACRES OUT OF NO. 060-016-0-00-007-01		
															0.848 ACRE OUT OF NO. 060-016-0-00-009-00		
	GRAND TOTAL					115.510	2.780	9.653	2.379	7.274	YES	105.456	–		REMOVE 1 STY. FR. RES.		
161T		36,37						0.517	–	0.517					TO CONSTRUCT & REMOVE TEMP.		
															RUN AROUND		
															EASEMENT TO THE DAYTON POWER		
															& LIGHT CO. – D.B. 117, PG. 320		
															EASEMENT TO BOARD OF FAYETTE		
															CO. COMMISSIONERS – D.B. 184, PG. 215		
162WD	TERRY GARNER	35	34	749	060-015-0-00-043-02	2.430	0.242	0.156	0.099	0.057	NO		2.131				
163WD	MICHAEL J. GARNER	35	41	692	060-015-0-00-043-01	2.430	0.202	0.367	0.202	0.165	NO		2.063				
164WL	DAVID L. GARNER	35	37	1019	060-015-0-00-043-03	(1.215deed)	0.160	0.096	0.056	0.040	NO						
						(1.226calc)											
164WD		35						1.130	0.104	1.026	NO						
	GRAND TOTAL					(1.226calc)	0.160	1.226	0.160	1.066	NO	–	–		TOTAL TAKE		
165WL	LEENORA MORROW, LEENORA B. MORROW	35	98 / 103	515 / 709	060-015-0-00-041-00	4.490	0.431	1.037	0.431	0.606	NO	–	3.453		0.011 ACRE OUT OF NO. 060-015-0-00-041-00		
					060-015-0-00-042-00										1.026 ACRES OUT OF NO. 060-015-0-00-042-00		
165T		35						0.381	–	0.381					TO CONSTRUCT RESIDENTIAL DRIVE		
															EASEMENT TO THE OHIO BELL		
															TELEPHONE CO., DB 175, PAGE 856		
166	CARL H. JAMES & BERNICE I. JAMES	35	170	423		171.522								STATE	NO R/W TAKE REQUIRED		

NOTE: UNDER NO CIRCUMSTANCES ARE TEMPORARY EASEMENTS ACQUIRED FOR THE PURPOSE OF STRUCTURAL REMOVAL TO BE USED FOR STORAGE OF MATERIAL OR EQUIPMENT BY THE CONTRACTOR. UPON COMPLETION OF THE WORK REQUIRED FOR SUCH REMOVAL AND SUBSEQUENT RECLAMATION, THE EASEMENT SHALL BE VACATED IMMEDIATELY.

NOTE: ALL TEMPORARY PARCELS TO BE OF 18 MONTHS DURATION.

REV.	DATE	DESCRIPTION
WGY	12-27-99	REVISED PARCEL 160WL ADDED PARCEL 160A WL
WGY	11-1-99	REVISED PARCEL 160 REMOVED PARCELS 160WD AND 160T
WGY	9-30-99	REVISED PARCELS 162, 163, AND 164

COMPLETION DATE: 01-08-99

SUMMARY OF ADDITIONAL RIGHT OF WAY PARCELS 160 – 166

FAY-35-2.57

10/40

492/522

C:\3559\RWSUM02

TOTAL NUMBERS OF

_____ OWNERSHIPS
_____ TOTAL TAKES
_____ OWNERSHIPS WITH STRUCTURES INVOLVED
_____ OWNERSHIPS WITH "P" ITEMS

GRANTEE:
ALL RIGHT OF WAY ACQUIRED IN THE NAME
OF STATE OF OHIO UNLESS OTHERWISE SHOWN.

P I D NO. 9078

ALL AREAS IN ACRES

NET RESIDUE = RECORD AREA – TOTAL P.R.O. – NET TAKE

PARCEL NO.	OWNER	SHEET NO.	OWNERS RECORD BOOK	OWNERS RECORD PAGE	AUDITOR'S PARCEL	RECORD AREA	TOTAL P.R.O.	GROSS TAKE	P.R.O. IN TAKE	NET TAKE	STRUC-TURE	NET RESIDUE LEFT	NET RESIDUE RIGHT	TYPE FUND	REMARKS AND PERSONALTY	AS ACQUIRED BOOK	AS ACQUIRED PAGE
167WL	LARRY L. MAXEL & VIRGINIA M. MAXEL	20,21	142 148	398 431	060-015-0-00-039-00	345.146	3.796	24.692	1.733	22.959	NO			STATE	18.288 ACRES OUT OF NO. 060-015-0-00-038-00		
		35,36	162	598	060-015-0-00-040-00										3.077 ACRES OUT OF NO. 060-015-0-00-039-00		
					060-015-0-00-038-00										0.326 ACRE OUT OF NO. 060-015-0-00-040-00		
					030-002-0-00-017-00										3.001 ACRES OUT OF NO. 030-002-0-00-017-00		
167WD		38			030-002-0-00-018-00			0.082	0.046	0.036	NO				NO. 030-002-0-00-018-00		
167WD1		38						1.419	0.353	1.066	NO				NO. 030-002-0-00-018-00		
	GRAND TOTAL					345.146	3.796	26.193	2.132	24.061	NO	5.867	311.422				
167T		38						0.012	–	0.012					TO CONSTRUCT FIELD DRIVE		
167T1		38						0.061	–	0.061					TO CONSTRUCT FIELD DRIVE		
															EASEMENT TO THE DAYTON POWER		
															& LIGHT CO. – D.B. 117, PG. 322		
167E		21,36						5.867	–	5.867					LANDLOCKED AREA – 5.867 ACRES		
168	NOT USED																
169	NOT USED																
170	JEFFERSON–JASPER TOWNSHIP CEMETERY														NO R/W TAKE REQUIRED		
171WD	IVA I. PEEL & EUGENE L. PEEL	38	159	109	030-002-0-00-019-00	8.225	0.127	0.333	0.127	0.206	NO	7.892	–				
171T		38						0.074	–	0.074					TO CONSTRUCT A RESIDENTIAL DRIVE		
172WL	RUTH ANN PERRILL SHERIDAN (1/2)	21,22,23	163	55	030-002-0-00-020-00	268.880	4.310	41.145	0.360	40.785	YES				LANDLOCKED AREA – 32.863 ACRES		
	JEAN PERRILL TELLER (1/4)	24,25,26	122	193											REMOVE 5 SHEDS		
172WD	JEAN PERRILL YOUNG (1/4)	38,39						3.646	0.958	2.688	NO						
172WD-1		39,40						4.999	1.517	3.482	NO						
	GRAND TOTAL					268.880	4.310	49.790	2.835	46.955	NO	105.570	112.045				
172T		38						0.039	–	0.039					TO CONSTRUCT A RESIDENTIAL DRIVE		
172T-1		38						0.271	–	0.271					TO CONSTRUCT A FIELD DRIVE		
172X		25,26						1.068	–	1.068					TO CONSTRUCT AND MAINTAIN		
															DRAINAGE CHANNEL		
															EASEMENT TO CNG TRANSMISSION		
															CORP. – D.B. 115, PG. 291		
172U		504						0.164		0.164					ACQUIRE IN NAME OF CNG		
172U1		504,521						1.251		1.251					TRANSMISSION CORP.		
172U2		521						0.289		0.289							
173WL	MEDRITH CREAMER	24	173	669	030-002-0-00-022-00	133.40	4.863	1.903	–	1.903	NO	126.634	–				
174	NOT USED																
175WL	JAMES K. HARRIS &	26	102	449	030-003-0-00-010-00	152.25	4.041	6.978	–	6.978	NO	138.248	2.983		LANDLOCKED AREA – 2.983 ACRES		
	IRENE CREAMER HARRIS																
														STATE			

NOTE: UNDER NO CIRCUMSTANCES ARE TEMPORARY EASEMENTS ACQUIRED FOR THE PURPOSE OF STRUCTURAL REMOVAL TO BE USED FOR STORAGE OF MATERIAL OR EQUIPMENT BY THE CONTRACTOR. UPON COMPLETION OF THE WORK REQUIRED FOR SUCH REMOVAL AND SUBSEQUENT RECLAMATION, THE EASEMENT SHALL BE VACATED IMMEDIATELY.

NOTE: ALL TEMPORARY PARCELS TO BE OF 18 MONTHS DURATION.

REV.	DATE	DESCRIPTION
WGY	1-31-00	ADDED PARCELS 172U,172U1,172U2
WGY	11-1-99	ADDED PARCEL 167E

COMPLETION DATE: 01-08-99

SUMMARY OF ADDITIONAL RIGHT OF WAY PARCELS 167 – 175

FAY-35-2.57

C:\3590\RWSUM03

TOTAL NUMBERS OF

_____ OWNERSHIPS
_____ TOTAL TAKES
_____ OWNERSHIPS WITH STRUCTURES INVOLVED
_____ OWNERSHIPS WITH "P" ITEMS

GRANTEE:
ALL RIGHT OF WAY ACQUIRED IN THE NAME
OF STATE OF OHIO UNLESS OTHERWISE SHOWN.

P I D NO. 9078

ALL AREAS IN ACRES

NET RESIDUE = RECORD AREA – TOTAL P.R.O. – NET TAKE

PARCEL NO.	OWNER	SHEET NO.	OWNERS RECORD BOOK	OWNERS RECORD PAGE	AUDITOR'S PARCEL	RECORD AREA	TOTAL P.R.O.	GROSS TAKE	P.R.O. IN TAKE	NET TAKE	STRUC-TURE	NET RESIDUE LEFT	NET RESIDUE RIGHT	TYPE FUND	REMARKS AND PERSONALTY	AS ACQUIRED BOOK	AS ACQUIRED PAGE
176WL	GLENN COX, MORMAN COX, AND VIRGIL COX	26,27	160/160	978/980	030-003-0-00-006-00	233.356	5.372	27.352	0.580	26.772	NO	75.546	125.666	STATE	9.989 ACRES IN THIRD TRACT		
	AS TRUSTEES OF THE MARGARET COX	28,29												▲	17.363 ACRES IN FIRST TRACT		
	TRUST, DATED FEBRUARY 9, 1987.																
	GLENN COX, MORMAN COX, AND VIRGIL COX																
	AS TRUSTEES OF THE HAROLD COX TRUST,																
	DATED FEBRUARY 9, 1987.																
177WL	FONDA GAY FICHTHORN	29,30	182	27	030-006-0-00-003-00	190.100	0.448	13.166	–	13.166	NO		167.363		LANDLOCKED AREA – 9.123 ACRES		
177E								9.123		9.123							
	TOTAL							22.289		22.289			167.363				
178WL	THEODORE D. CRAIG AND DAVID W. CRAIG	30,31	145/150	649/785	030-003-0-00-003-00	78.970	0.472	13.209	–	13.209	YES	62.452	2.837		LANDLOCKED AREA – 2.837 ACRES		
		32													REMOVE POLE BARN		
178X		32						1.481	–	1.481					TO CONSTRUCT AND MAINTAIN		
															DRAINAGE CHANNEL		
															EASEMENT TO TEXAS EASTERN		
															TRANSMISSION CORPORATION		
179	McDONALDS FARMS, INC.	30,31	141	384	030-006-0-00-004-00	70.75									NO R/W TAKE REQUIRED		
		32													EASEMENT TO TEXAS EASTERN		
															TRANSMISSION CORPORATION		
															D.B. 81, PG. 3		
														▼			
														STATE			

NOTE: UNDER NO CIRCUMSTANCES ARE TEMPORARY EASEMENTS ACQUIRED FOR THE PURPOSE OF STRUCTURAL REMOVAL TO BE USED FOR STORAGE OF MATERIAL OR EQUIPMENT BY THE CONTRACTOR. UPON COMPLETION OF THE WORK REQUIRED FOR SUCH REMOVAL AND SUBSEQUENT RECLAMATION, THE EASEMENT SHALL BE VACATED IMMEDIATELY.

NOTE: ALL TEMPORARY PARCELS TO BE OF 18 MONTHS DURATION.

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 177 E
COMPLETION DATE:	01-08-99	

SUMMARY OF ADDITIONAL RIGHT OF WAY PARCELS 176 – 179

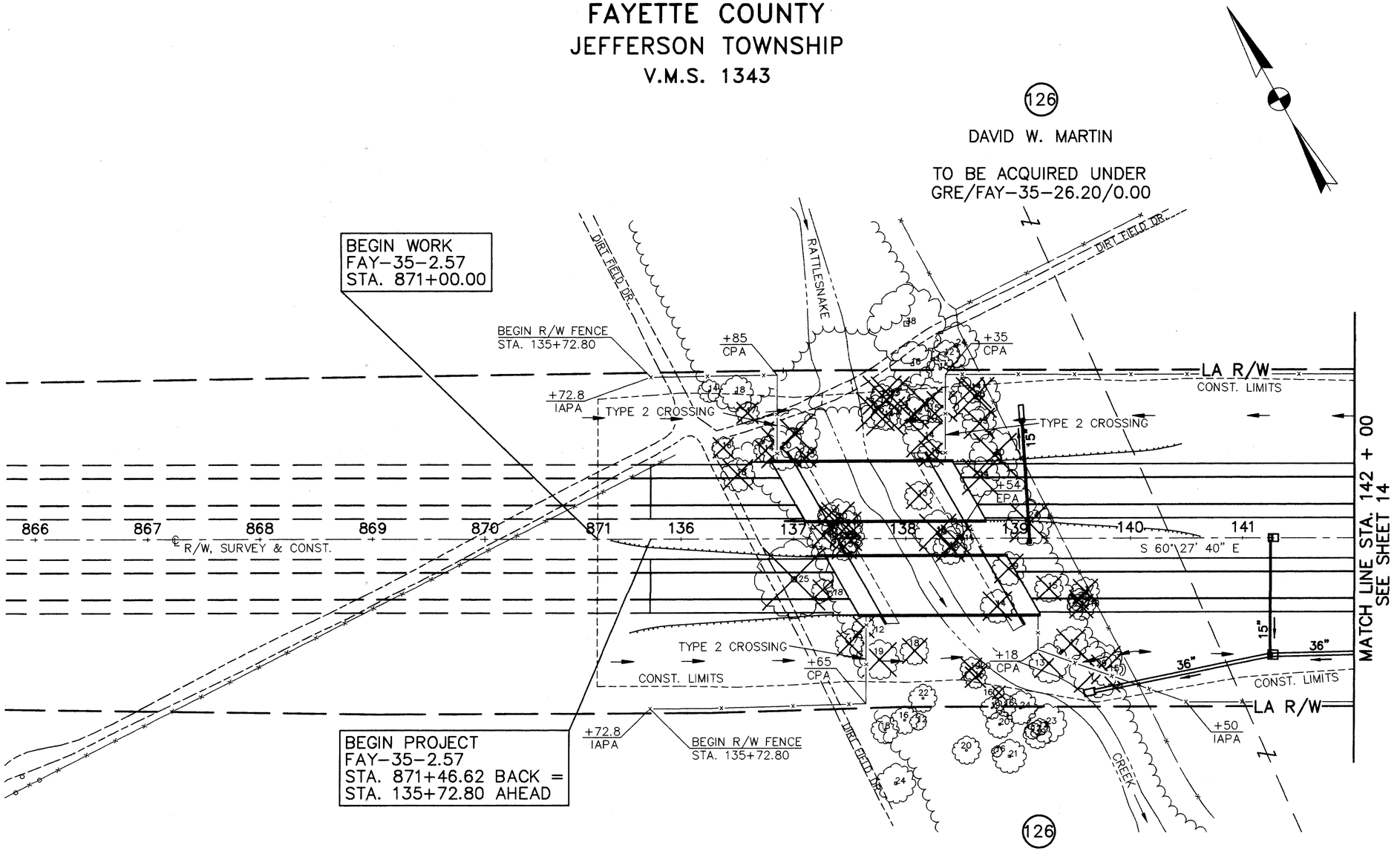
FAY-35-2.57

12/40

494/522

C:\3590\RWSUM04

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343



FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 1294 L.F.
CARRIED TO SHEET NO.42

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343

NOTE: ITEM 607-FENCE TYPE 47 QUANTITY HAS BEEN CARRIED TO SHEET NO.42 FROM EACH R/W PLAN SHEET NO.495 THRU NO.522.

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

HORIZONTAL SCALE IN FEET
0 25 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 866 + 00 TO STA. 142 + 00

FAY-35-2.57

13/40

495/522

Q:\3590\MLRW01

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343

(126)

DAVID W. MARTIN

TO BE ACQUIRED UNDER
GRE/FAY-35-26.20/0.00

CONC. POST

PASTURE

(150)

LESTER W. MARTIN, TRUSTEE OF THE LESTER W. MARTIN TRUST, DATED THE 15th DAY OF MAY, 1990. JOAN B. MARTIN, TRUSTEE OF THE JOAN B. MARTIN TRUST, DATED THE 15th DAY OF MAY, 1990.

BEGIN R/W ACQUISITION
FAY-35-2.57
STA. 151+68.60

BEGIN LIMITED ACCESS
LEFT STA. 154+41.32

DB 176 PG 917
DB 176 PG 923
TRACT NO. 1
13.587 Ac.
060-011-0-00-001-01

154+41.32
150.00' LT.

+00 IAPA — LA R/W — +60 IAPA — +20 IAPA — LA R/W 311.95'

S 60°27'40" E

15" TILE

CONST. LIMITS

N 03°18'39" E 142.43'

155+04.26
22.23' LT.

(150 WL)

279.87'

SEE SHEET 13
MATCH LINE STA. 142 + 00

143 144 145 146 147 148 149 150 151 152 153 154 155

℄ R/W, SURVEY & CONST.

S 60° 27' 40" E

15"

328.57'

154+60.93, ℄

MATCH LINE STA. 156 + 00
SEE SHEET 15

36" 36" 36" 36" 36"

CONST. LIMITS

N 87°37'26" W 657.15' (TOTAL)

(151 WL)

+50 IAPA

CONST. LIMITS

LA R/W — +00 IAPA — N 60°27'40" W — LA R/W 2206.89' (TOTAL)

151+68.60
150.00' RT.

107.48'

PARCEL IV

CULTIVATED

(151)

JOAN ALLEN MINOR

DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00

(126)

DAVID W. MARTIN

TO BE ACQUIRED UNDER
GRE/FAY-35-26.20/0.00

V.M.S. 1343 / V.M.S. 1361 — N 87°37'26" W

BEGIN LIMITED ACCESS
RIGHT STA. 151+68.60

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

LEGEND
● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2800 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0 50 100

P I D NO. 9078

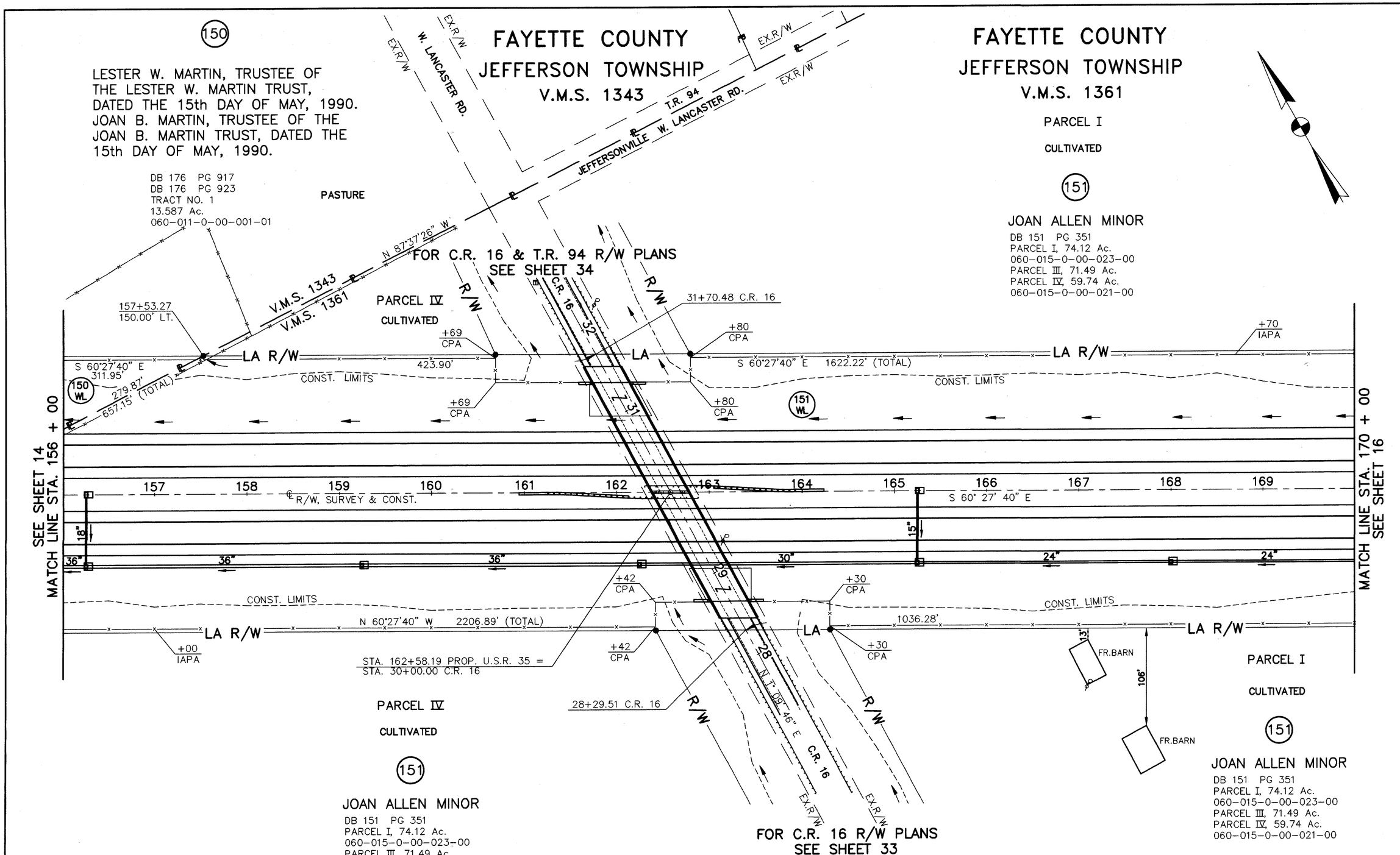
RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 142 + 00 TO STA. 156 + 00

FAY-35-2.57

14 / 40

496
522

Q:\3590\MLRW02



NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS

LEGEND
● IRON PIN SET

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2832 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 156 + 00 TO STA. 170 + 00

FAY-35-2.57

15/40

497/522

Q:\3590\MLRW03

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

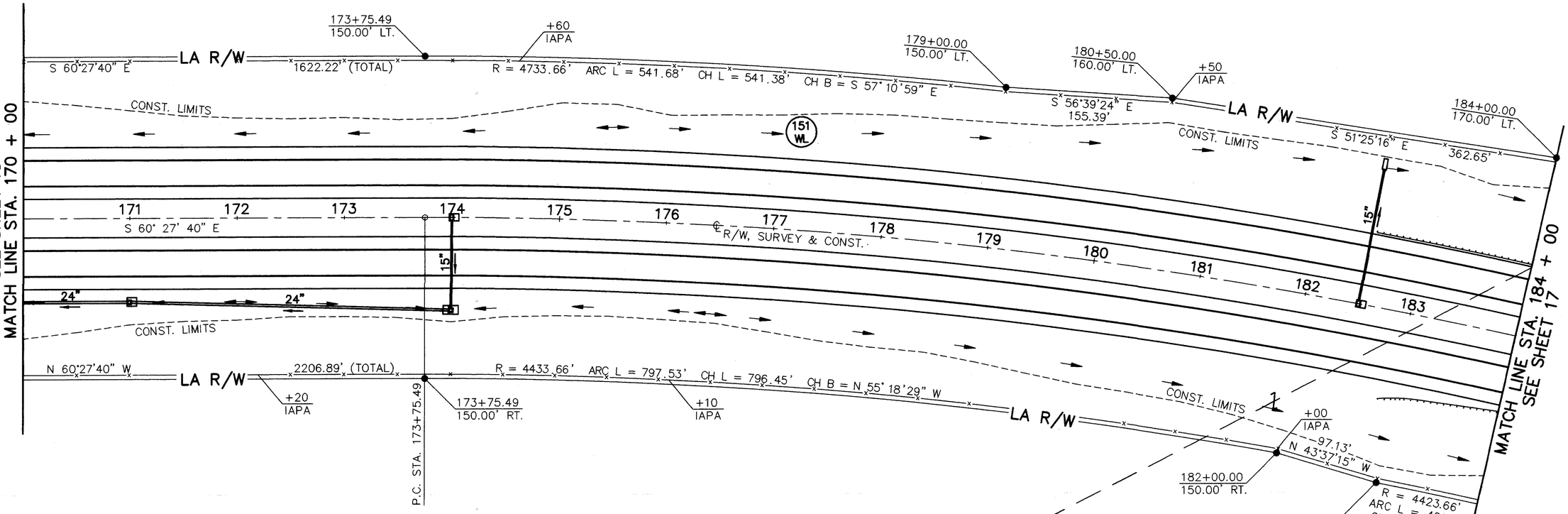
(151)

JOAN ALLEN MINOR
DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00

CULTIVATED

PARCEL I

CURVE DATA
P.I. STA. 196+74.95
P.C. STA. 173+75.49
P.T. STA. 216+38.09
Δ = 53° 16' 57" RT.
Dc= 1° 15' 00"
R = 4583.66'
T = 2299.46'
L = 4262.60'
E = 544.44'



PARCEL III

PARCEL I

CULTIVATED

(151)

JOAN ALLEN MINOR
DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2802 L.F.
CARRIED TO SHEET NO.42

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

HORIZONTAL SCALE IN FEET
0 25 50 100

P I D NO. 9078

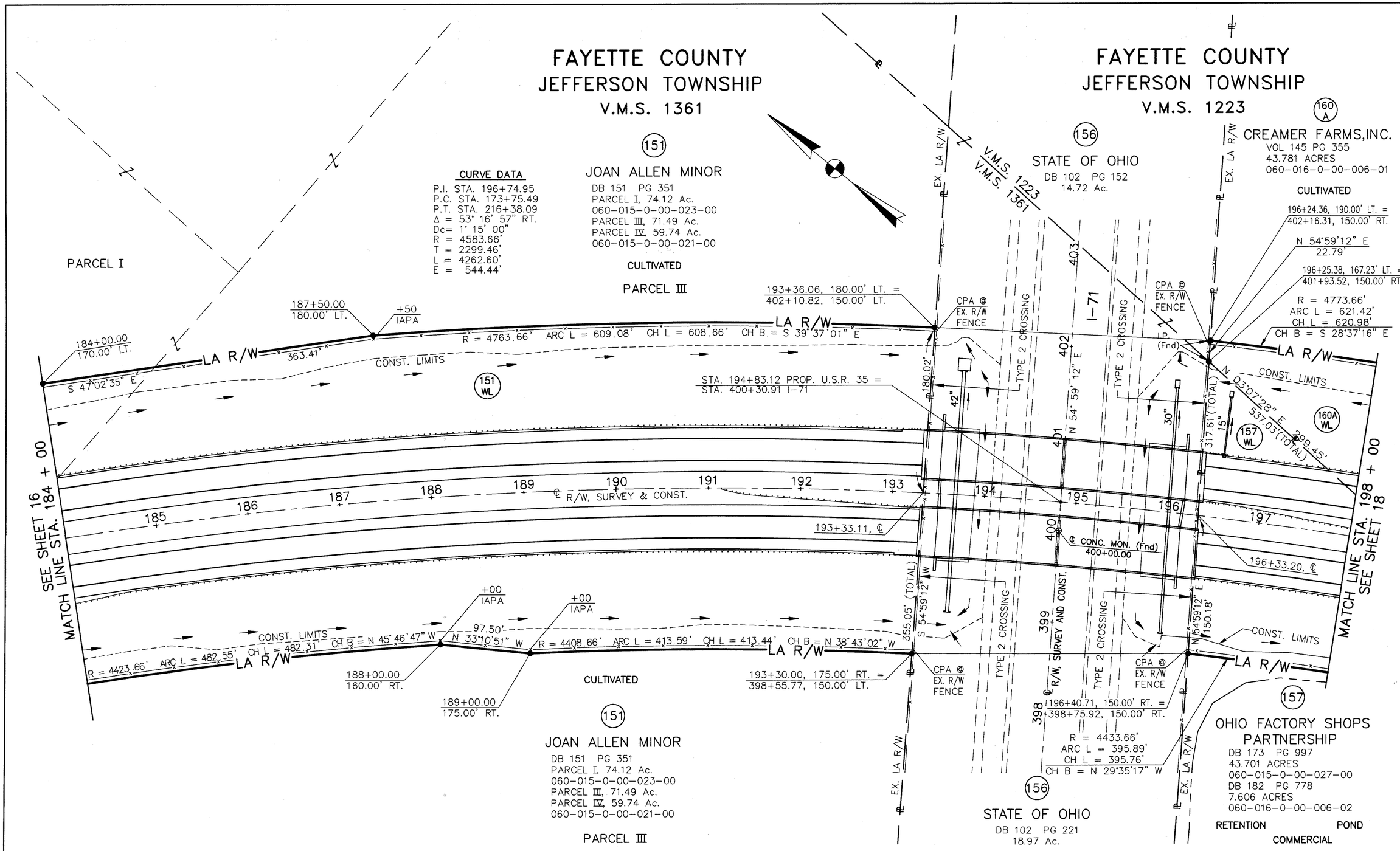
RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 170 + 00 TO STA. 184 + 00

FAY-35-2.57

16/40

498/522

Q:\3590\MLRW04



151
JOAN ALLEN MINOR
DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00
PARCEL III

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY RECORDS
FAY-1-2.35

LEGEND
● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2666 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 160A WL

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 184 + 00 TO STA. 198 + 00

FAY-35-2.57

17/40

499/522

Q:\3590\MLRW05

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

CREAMER FARMS, INC. AND
DWIGHT S. CREAMER, TRUSTEE, UNDER THE
FORREST W. CREAMER REVOCABLE TRUST
DATED SEPTEMBER 17, 1990
DB 145 PG 357
DB 179 PG 693
TRACT 1, 36.48 ACRES
060-016-0-00-004-00
TRACT 2, 62.135 ACRES
060-016-0-00-006-00
TRACT 4, 16.582 ACRES
060-015-0-00-037-00

CURVE DATA
P.I. STA. 196+74.95
P.C. STA. 173+75.49
P.T. STA. 216+38.09
Δ = 53° 16' 57" RT.
Dc= 1° 15' 00"
R = 4583.66'
T = 2299.46'
L = 4262.60'
E = 544.44'

CULTIVATED

CREAMER FARMS, INC.
DB 145 PG 355
43.781 ACRES
060-016-0-00-006-01

160 A

+50 IAPA

202+21.04
190.00' LT.

R

12"TILE

TYPE 2 CROSSING

+70 IAPA

15"TILE

CULTIVATED

+90 IAPA

LA R/W

R = 4773.66' ARC L = 621.42' CH L = 620.98' CH B = S 28°37'16" E

R = 4773.66' ARC L = 1475.78' CH L = 1469.91' CH B = S 16°02'07" E

LA R/W

CONST. LIMITS

CONST. LIMITS

345.42'(TOTAL)

209.17'

160 WL

160A WL

TRACT 2

MATCH LINE STA. 198 + 00
SEE SHEET 17

MATCH LINE STA. 212 + 00
SEE SHEET 19

198+69.39, ℄

201+35.31

S 89°18'08" W

299.45'

537.03'(TOTAL)

198 199 200 201 202 203 204 205 206 207 208 209 210 211

℄ R/W, SURVEY & CONST.

15"

36.25'

237.58'
S 03°07'28" W

200+75.56
122.83' RT.

201+09.19
141.73' RT.
I.P.(Fnd)

R = 2510.00'
ARC L = 264.04'
CH L = 263.92'
CH B = N 22°19'52" W

203+81.67
152.12' RT., I.P. (Fnd)

206+68.77
160.70' RT., I.P. (Fnd)

R = 4423.66' ARC L = 626.95' CH L = 626.42'
CH B = N 15°15'28" W

157 WL

+50 IAPA

CONST. LIMITS

LA R/W

244.36' N 24°34'42" W 320.68'(TOTAL)

N 19°19'04" W 277.39'

+81.67 CPA

ABANDON

CULTIVATED

CONST. LIMITS

LA R/W

+40 IAPA

200+50.00
150.00' RT.

S 03°33'03" W
37.73'

201+28.88
152.61' RT.

N 03°33'03" E

EX. C.B.

18" TILE

EX. C.B.

R = 4433.66'
ARC L = 395.89'
CH L = 395.76'
CH B = N 29°35'17" W

157

OHIO FACTORY SHOPS
PARTNERSHIP
DB 173 PG 997
43.701 ACRES
060-015-0-00-027-00
DB 182 PG 778
7.606 ACRES
060-016-0-00-006-02

RETENTION POND

COMMERCIAL

V.M.S. 1223
V.M.S. 1361

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

FUTURE BUILDING

COMMERCIAL

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

RETENTION POND

LEGEND
● IRON PIN SET
○ IRON PIN FOUND

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2811 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 160A WL
WGY	12-27-99	REVISED PARCEL 160 WL

COMPLETION DATE: 01-08-99

HORIZONTAL SCALE IN FEET
0 25 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 198 + 00 TO STA. 212 + 00

FAY-35-2.57

18 / 40

500 / 522

Q:\3590\WLRW06

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

TRACT 2

CURVE DATA
P.I. STA. 196+74.95
P.C. STA. 173+75.49
P.T. STA. 216+38.09
Δ = 53° 16' 57" RT.
Dc= 1° 15' 00"
R = 4583.66'
T = 2299.46'
L = 4262.60'
E = 544.44'

CULTIVATED

213+71.46, I.P. (Fnd)
190.00' LT.

LA R/W

R = 4773.66' ARC L = 1479.78' CH L = 1469.91' CH B = S 16° 02' 07" E

216+38.09, I.P. (Fnd)
190.00' LT.

S 07°10'43" E 71.22'

+09 IAPA

217+09.31, I.P. (Fnd)
190.00' LT.

S 00°39'22" W 293.43'

220+00.00
150.00' LT.

+00 IAPA

CONST. LIMITS

160 WL

(160)

CREAMER FARMS, INC. AND
DWIGHT S. CREAMER, TRUSTEE, UNDER THE
FORREST W. CREAMER REVOCABLE TRUST
DATED SEPTEMBER 17, 1990

DB 145 PG 355
DB 179 PG 693
TRACT 1, 36.48 ACRES
060-016-0-00-004-00
TRACT 2, 62.135 ACRES
060-016-0-00-006-00
TRACT 4, 16.582 ACRES
060-015-0-00-037-00

TRACT 1

TRACT 4

CULTIVATED

DIRT FIELD DR.

LA R/W

S 07°10'43" E 1131.17'

+70 IAPA

CONST. LIMITS

TRACT 2 | TRACT 1 | TRACT 1 | TRACT 4

SEE SHEET 18
MATCH LINE STA. 212 + 00

213 214 215 216 217 218 219 220 221 222 223 224 225

15" 30" 36" 24"

℄ R/W, SURVEY & CONST.

S 7° 10' 43" E

MATCH LINE STA. 226 + 00
SEE SHEET 20

CONST. LIMITS

CONST. LIMITS

LA R/W

N 11°06'45" W 308.90'

+38 IAPA

N 07°10'43" W 1427.44'

+60 IAPA

LA R/W

213+18.48
160.44' RT.
I.P.(Fnd)

216+38.09
150.00' RT.

P.T. STA. 216+38.09

R = 4423.66'
ARC L = 626.95'
CH L = 626.42'
CH B = N 15°15'28" W

CULTIVATED

(154)

COLORADO LAND COMPANY, LTD.

VOL 50 PG 889
VOL 50 PG 894
18.355 ACRES
060-016-0-00-004-01

(157)

RETENTION POND

LEGEND
● IRON PIN SET
○ IRON PIN FOUND

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2806 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	12-27-99	REVISED PARCEL 160 WL
WGY	11-1-99	ADDED PARCEL 154

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 212 + 00 TO STA. 226 + 00

FAY-35-2.57

19/40

501/522

Q:\3590\MLRW07

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

THIRD TRACT

CULTIVATED

I.P. (Fnd)

160

CREAMER FARMS, INC. AND
DWIGHT S. CREAMER, TRUSTEE, UNDER THE
FORREST W. CREAMER REVOCABLE TRUST
DATED SEPTEMBER 17, 1990

DB 145 PG 357
DB 179 PG 693
TRACT 1, 36.48 ACRES
060-016-0-00-004-00
TRACT 2, 62.135 ACRES
060-016-0-00-006-00
TRACT 4, 16.582 ACRES
060-015-0-00-037-00

SECOND TRACT

161

82+00.00
251.78' LT.

35' EASEMENT
D.B. 184, PG. 215
BOARD OF FAYETTE
COUNTY COMMISSIONERS

MATCH SEE SHEET 36 STA. 82 + 00

S 45°57'31" E 172.35'

R/W

8"SAN

161 WD

161 WL

80+35.83
290.06' LT.

UGT STATION

S 03°15'46" W

191.41'

345.12'

160 WD-1

4" SAN. FORCE MAIN

CONST. LIMITS

S 59°22'43" E

231+38.47, 145.35' LT. =
76+23.47, 120.00' LT.

115

LA

N 59°22'43" W

301.21'

148.62'

S 03°15'46" W

RT. SERVICE RD. 3

10775.17'

81+23.41
120.67' LT.

EX. R/W

I.P. (Fnd)

81

80

℄ R/W, SURVEY & CONST.

EX. U.S.R. 35

UGT

81+93.19
10.37' RT.

167

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 142 PG 398
162.967 Ac.
030-002-0-00-017-00
30.339 Ac.
060-015-0-00-038-00
DB 162 PG 598
FIRST TRACT - 26.93 Ac.
060-015-0-00-039-00
SECOND TRACT - 0.320 Ac.
060-015-0-00-040-00

CULTIVATED

CURVE DATA
P.I. STA. 251+01.07
P.C. STA. 229+44.25
P.T. STA. 270+37.24
Δ = 60° 01' 50" LT.
Dc= 1° 28' 00"
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'

229+16.03, 150.00' LT. =
76+91.63, 290.00' LT.

EX. M.H.

N 07°10'43" W

LA R/W

215.15'

LA

S 07°10'43" E

1131.17'

CONST. LIMITS

TRACT 4

+39.4
CPA

STA. 231+71.97 PROP. U.S.R. 35 =
STA. 77+35.13 EX. U.S.R. 35

79

N 59°22'43" W
192.63'

EX. U.S.R. 35

15"

UGT

EX. R/W

81+93.19
10.37' RT.

MATCH LINE STA. 240 + 00
SEE SHEET 21

E

CULTIVATED

239

238

237

236

235

234

233

232

231

230

229

227

228

P.C. STA. 229+44.25

8"SAN

160 WL

TEMP. ROAD

℄ R/W, SURVEY & CONST.

78

646.80'

24"

18"

S 07° 10' 43" E

MATCH LINE STA. 226 + 00
SEE SHEET 19

24"

228+79.49, 150.00' RT. =
74+32.18, 135.00' LT.

CONST. LIMITS

8"SAN

EX. M.H.

1427.44'

N 07°10'43" W

LA R/W

179.49'

110'

792.18'

S 59°22'43" E

75+61.62
73.50' LT.

186.04'

N 89°13'26" W

96.42'

N 03°05'21" E

16

227+00.00, 150.00' RT. =
73+22.17, 276.82' LT.

TEMP. CONST. LIMITS

PROP. M.H.

TEMP. R/W

154 T

+83.6
CPA

OVERLAP AREA

75

75+46.21
12.00' RT.

76+06.18
12.00' RT.

RAMP C

RAMP CC

N 50°21'12" E
65.68'

73+00.00
215.00' LT.

154 WL

74

171.80'

UGT

74+12.60
12.00' RT.

EX. R/W

EX. DIRT FIELD DR.

856.26'

S 30°25'28" E

852.21'

MATCH LINE STA. 73 + 00
SEE SHEET 35

15"

30"

CONST. LIMITS

LA R/W

1147.91'

N 30°25'28" W

+50
IAPA

167 WL

68 KVA

240+00.00
180.00' RT.

50' EASEMENT
DB 117 PG 322
THE DAYTON POWER
& LIGHT COMPANY

154

COLORADO LAND COMPANY, LTD.

VOL 50 PG 889
VOL 50 PG 894
18.355 ACRES
060-016-0-00-004-01

167

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 142 PG 398
162.967 Ac.
030-002-0-00-017-00
30.339 Ac.
060-015-0-00-038-00
DB 162 PG 598
FIRST TRACT - 26.93 Ac.
060-015-0-00-039-00
SECOND TRACT - 0.320 Ac.
060-015-0-00-040-00

CULTIVATED

NOTE
THE EXISTING R/W WIDTH AND LOCATION
WERE DETERMINED USING:

COUNTY RECORDS
S.H. 29 - SEC. Gpt & Hpt
FAY-35-3.31

LEGEND

- ● IRON PIN SET
- ○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2389 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	12-27-99	REVISED PARCEL 160
WGY	11-1-99	ADDED PARCEL 154

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0, 50, 100

P I D NO. 9078

RIGHT-OF-WAY PLAN PROP. U.S.R. 35
STA. 226 + 00 TO STA. 240 + 00

FAY-35-2.57

20 / 40

502 / 522

Q:\3590\WLRW08

SEE SHEET 36

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

LANDLOCKED AREA
5.867 ACRES

(167)

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 142 PG 398
162.967 Ac.
030-002-0-00-017-00
30.339 Ac.
060-015-0-00-038-00
DB 162 PG 598
FIRST TRACT - 26.93 Ac.
060-015-0-00-039-00
SECOND TRACT - 0.320 Ac.
060-015-0-00-040-00

CULTIVATED

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

WOODS

SEE SHEET 36 MATCH LINE STA. 240 + 00

SEE SHEET 20 MATCH LINE STA. 240 + 00

SEE SHEET 22 MATCH LINE STA. 254 + 00

LA R/W

S 09°50'41" W

242+00.00
325.00' LT.

316.52'

50' EASEMENT
DB 117 PG 322
THE DAYTON POWER
& LIGHT COMPANY

CONST. LIMITS

69 KVA

241 242 243 RAMP D 244 245 246 247 248 249 250 251 252

18"

+00
IAPA

S 10°01'24" E 393.30'

246+00.00
200.00' LT.

S 29°09'23" E 241.01'

248+52.79
182.51' LT.

302.96'

61.95'

248+73.98
210.21' LT.

117.93'

N 89°20'39" W

I.P. (Fnd)

249+49.42
304.78' LT.

V.M.S. 1223 / V.M.S. 1278

JEFFERSON TWP. / JASPER TWP.

(161)

S 03°11'54" W 752.39'

(167E)

WETLAND

249+17.30
175.51' LT.

53.86'

S 29°09'23" E 79.79'

250+00.00
165.00' LT.

S 42°56'21" E

861.94'

LA R/W

CONST. LIMITS

(172 WL)

+00
IAPA

N 04°19'20" E
262.69'

(167 WL)

R/W, SURVEY & CONST.

247+22.58, ℄

251+17.32, ℄

R = 497.73'

(TOTAL)
235.04'

R = 4071.53'
ARC L = 384.28'
CH. L. = 384.14'
CH. B. = N 44°09'33" W

RAMP C

240 241 242 243 244 245 246

CONST. LIMITS

LA R/W

N 28°31'09" W 643.83'

R = 4071.53' ARC L = 692.27' CH L = 691.44' CH B = N 36°35'04" W

240+00.00
180.00' RT.

+00
IAPA

+17
IAPA

246+17.08
165.00' RT.

252+81.29
165.00' RT.

WOODS

JEFFERSON TWP. / JASPER TWP.

V.M.S. 1223 / V.M.S. 1278

(167)

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 142 PG 398
162.967 Ac.
030-002-0-00-017-00
30.339 Ac.
060-015-0-00-038-00
DB 162 PG 598
FIRST TRACT - 26.93 Ac.
060-015-0-00-039-00
SECOND TRACT - 0.320 Ac.
060-015-0-00-040-00

CULTIVATED

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

CURVE DATA

P.I. STA. 252+01.07
P.C. STA. 229+44.25
P.T. STA. 270+37.24
Δ = 60° 01' 50" LT.
Dc= 1° 28' 00"
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'

LEGEND

● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2711 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	11-1-99	ADDED PARCEL 167E

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 240 + 00 TO STA. 254 + 00

FAY-35-2.57

21/40

503/522

Q:\3590\WLRW09

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

CURVE DATA
P.I. STA. 252+01.07
P.C. STA. 229+44.25
P.T. STA. 270+37.24
Δ = 60° 01' 50" LT.
Dc= 1° 28' 00"
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'

(172)
RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)
DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

WOODS
CULTIVATED
DIRT FIELD DR.
SEE SHEET 21
MATCH LINE STA. 254 + 00
MATCH LINE STA. 268 + 00
SEE SHEET 23
S 42°56'21" E
LA R/W
861.94'
CONST. LIMITS
+00 IAPA
259+00.00 150.00' LT.
R = 3756.53' ARC L = 1093.57' CH L = 1089.71' CH B = S 58°52'10" E
266+74.27 369.34'LT.
S 84°09'13" E 391.75'
N 69°08'11" E 207.22'
(172 U1)
N 84°09'13" W 369.55'
267+32.53 293.75'LT.
R = 3756.53' ARC L = 90.00' CH L = 90.00' CH B = N 60°28'01"W
265+30.49 210.50'LT.
266+24.92 177.78'LT.
S 69°08'11" W 153.85'
S 29°59'28" W 27.78'
N 29°59'28" E 60.50'
266+24.34 150.00'LT.
265+30.74 150.00' LT.
+50 IAPA
(172 WL)
SHED
18"
15"
36"
26" GASLINE
255 256 257 258 259 260 261 262 263 264 265 266 267
℄ R/W, SURVEY & CONST.
+50 IAPA
CONST. LIMITS
+00 IAPA
R = 4071.53' LA R/W
ARC L. = 384.28'
CH. L. = 384.14'
CH. B. = N 44°09'33" W
256+50.00 165.00' RT.
N 53°27'11" W 156.76'
258+00.00 150.00' RT.
R = 4056.53' ARC L = 1284.74' CH L = 1279.38' CH B = N 58°08'10" W
266+18.57 150.00' RT.
R = 4056.53' ARC L. = 434.75' ARC L. = 434.54' CH. L. = N 64°08'20" W
265+31.89 150.00' RT.
(172 U)
R = 4056.53' ARC L = 90.00' CH L = 90.00' CH B = S 60°26'23"E
123.12' N 29°59'28" E
S 75°36'13" W 125.94'
S 29°59'28" W 35.69'
90'
265+32.32 273.12' RT.
266+17.94 185.68' RT.
CNG TRANSMISSION CORPORATION D.B. 115, PAGE 291

(172)
RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)
DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2801 L.F.
CARRIED TO SHEET NO..42

REV.	DATE	DESCRIPTION
WGY	1-31-00	ADDED PARCELS 172U,172U1

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 254 + 00 TO STA. 268 + 00

FAY-35-2.57

22 / 40

504 / 522

Q:\3590\MLRW10

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

CURVE DATA

P.I. STA. 252+01.07
P.C. STA. 229+44.25
P.T. STA. 270+37.24
Δ = 60° 01' 50" LT.
Dc= 1° 28' 00"
R = 3906.53'
T = 2256.82'
L = 4092.99'
E = 605.04'

EX.R/W

EX.R/W

90'

CNG TRANSMISSION CORPORATION D.B. 115, PAGE 291

FOR S.R. 729 R/W PLANS SEE SHEET 39

PIPELINE MARKER

℄ R/W, SURV. & CONST.

R/W

R/W

S.R.729

169

P.T.STA. 270+37.24

26" GASLINE

R = 3756.53'
ARC L. = 1093.57'
CH. L. = 108971'
CH. B. = S 58°52'10" E

270+37.24
150.00' LT.

+87
CPA

253.84'

+87
CPA

168+55.95

LA R/W

LA

CONST. LIMITS

STA. 273+36.81 PROP. U.S.R. 35 =
STA. 166+99.14 S.R. 729

+06
CPA

+06
CPA

172
WL

S 67°12'33" E 1514.25' (TOTAL)

+00
IAPA

LA R/W

CONST. LIMITS

12"TILE

15"

SEE SHEET 22
MATCH LINE STA. 268 + 00

269 270 271 272 273 274 275 276 277 278 279 280 281

℄ R/W, SURVEY & CONST.

S 67° 12' 33" E

MATCH LINE STA. 282 + 00
SEE SHEET 24

24"

4'CONC. PILLAR

+50
CPA

165+42.32

166

+10
CPA

CONST. LIMITS

CONST. LIMITS

R = 4056.53' ARC L. = 1284.74' CH. L. = 1279.38' CH. B. = N 58°08'10" W

R = 4056.53'
ARC L. = 434.75'
CH. L. = 434.54'
CH. B. = N 64°08'20" W

270+37.24
150.00' RT.

N 67°12'33" W 345.29'

+50
CPA

LA R/W

LA

2207.73' N 67°12'33" W 2553.02' (TOTAL)

+10
CPA

LA R/W

+50
IAPA

N 05°50'17" E

165

R/W

R/W

12"TILE

76"x48"

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

FOR S.R. 729 R/W PLANS SEE SHEET 39

EX.R/W

EX.R/W

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY RECORDS

LEGEND

● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2805 L.F.
CARRIE TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	1-31-00	ADDED DIMENSIONS FOR PARCEL172U

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 268 + 00 TO STA. 282 + 00

FAY-35-2.57

23/40

505/522

Q:\3590\WLRW11

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(172)
RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)
DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00
CULTIVATED

(173)
MEDRITH CREAMER
DB 173 PG 669
PARCEL I
67.27 Ac.
030-002-0-00-022-00
WOODS

LANDLOCKED AREA
32.863 Ac.

(172)
RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)
DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00
CULTIVATED

12"TILE

285+51.49
150.00' LT.
+00
IAPA

289+00.00
150.00' LT.

292+30.46
155.85' LT.
+00
IAPA

LA R/W
S 67°12'33" E 1514.25'
CONST. LIMITS
S 67°12'33" E 348.51'
S 68°13'25" E 330.51'
S 68°13'25" E 575.80'
LA R/W

(172 WL)
(173 WL)

S 05°43'40" W 156.91'
257.91' (TOTAL)
N 89°54'46" W 653.97' (TOTAL)
403.80'
24"
24"
OLD FIELD DR.

SEE SHEET 23
MATCH LINE STA. 282 + 00

283 284 285 286 287 288 289 290 291 292 293 294 295
℄ R/W, SURVEY & CONST.
S 67° 12' 33" E

MATCH LINE STA. 296 + 00
SEE SHEET 25

285+97.53, ℄
288+57.95, ℄
101.00'
250.17'

CONST. LIMITS
CONST. LIMITS

286+27.17
96.56' RT.

LA R/W
N 67°12'33" W 2553.02'
LA R/W

+00
IAPA
+50
IAPA

295+90.26
150.00' RT.

133.4'
1 STY
FR.RES.
FR.SHED
OLD FIELD DR.

CURVE DATA
P.I. STA. 310+76.19
P.C. STA. 295+90.26
P.T. STA. 324+98.06
Δ = 29° 04' 41" RT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'

R = 5579.58'
ARC L = 1627.50'
CH L = 1621.74'
CH B = N 58° 51' 10" W

P.C. STA. 295+90.26

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2800 L.F.

CARRIED TO SHEET NO.42

(172)
RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)
DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00
CULTIVATED

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

FR. PORT

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT-OF-WAY PLAN PROP. U.S.R. 35
STA. 282+ 00 TO STA. 296 + 00

FAY-35-2.57

Q:\3590\MLRW12

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

LANDLOCKED AREA
32.863 Ac.

V.M.S. 1342
V.M.S. 1278

307+59.18
690.00' LT.

MISSOURI DITCH

12"

S 85°04'46" E
80.00'

306+96.69
650.91' LT.

16"

13"

2-12"

CHANNEL EASE.
S 04°55'14" W 606.54'

CHANNEL EASE.
N 04°55'14" E 556.53'

1

(172 X)

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

MATCH LINE STA. 2+00

MATCH LINE STA. 2+00

JAMES K. HARRIS AND IRENE CREAMER HARRIS
PASTURE
(175)

CHANNEL EASE. V.M.S. 1342
S 04°55'14" W 606.54'

CHANNEL EASEMENT
N 04°55'14" E 556.53'

3

4

5

(172 X)

R = 5899.58'
ARC L = 94.34'
CH L = 94.34'
CH B = N 53°04'15" W

309+58.28
170.00' LT.

298+00.00
170.00' LT.

S 68°13'25" E

LA R/W

575.80'

CONST. LIMITS

+00
IAPA

R = 5899.58' ARC L 1286.99' CH L = 1284.44' CH B = S 58° 51' 44" E

+00
IAPA

15"TILE

6"TILE

12"TILE

LA R/W

CONST. LIMITS

+75
EPA

(172 WL)

15"

60"

SEE SHEET 24
MATCH LINE STA. 296 + 00

297 298 299 300 301 302 ℄ R/W, SURVEY & CONST. 303 304 305 306 307 308 309

MATCH LINE STA. 310 + 00
SEE SHEET 26

12"TILE

+50
IAPA

CONST. LIMITS

LA R/W

R = 5579.58' ARC L = 1627.50' CH L = 1621.74' CH B = N 58° 51' 10" W

+00
IAPA

LA R/W

CONST. LIMITS

R = 5579.58'
ARC L = 376.39'
CH L = 376.32'
CH B = N 52°25'45" W

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

CURVE DATA

P.I. STA. 310+76.19
P.C. STA. 295+90.26
P.T. STA. 324+98.06
Δ = 29° 04' 41" RT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'

LEGEND

● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2780 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 296 + 00 TO STA. 310 + 00

FAY-35-2.57

25/40

(507/522)

Q:\3590\MLRW13

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

SCALE IN FEET: 0, 50, 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 310 + 00 TO STA. 324 + 00

FAY-35-2.57

26 / 40

508
522

(175)

JAMES K. HARRIS AND
IRENE CREAMER HARRIS

DB 102 PG 449
FIRST TRACT
152.25 Ac.
030-003-0-00-010-00

PASTURE

CHANNEL EASEMENT
S 04°55'14" W
606.34'

R = 5899.58'
ARC L = 94.34'
CH L = 94.34'
CH B = N 53°04'15" W

310+49.90
170.00' LT.

+50
EPA

R = 5899.58'
ARC L = 154.55'
CH L = 154.55'
CH B = S 51°51'44" E

312+00.00
170.00' LT.

316+50.00
150.00' LT.

S 46°23'14" E 462.88'

R = 5879.58' ARC L = 667.65' CH L = 667.29' CH B = S 43°21'31" E

R = 5879.58'
ARC L = 202.62'
CH L = 202.61'
CH B = S 39°07'06" E

323+00.62
150.00' LT.

CULTIVATED

(172 X)

LA R/W

CONST. LIMITS

+00
IAPA

+50
IAPA

LA R/W

CONST. LIMITS

R = 5899.58'
ARC L = 1286.99'
CH L = 1284.44'
CH B = S 58°51'44" E

S 04°55'14" W
202.72'

430.67' (TOTAL)
212.35'

(175 WL)

SEE SHEET 25
MATCH LINE STA. 310 + 00

MATCH LINE STA. 324 + 00

18"

30"

15"

311 312 313 314 315 316 317 318 319 320 321 322 323

℄ R/W, SURVEY & CONST.

311+58.73, ℄

321+52.24, ℄

R = 5579.58'
ARC L = 1627.50'
CH L = 1621.74'
CH B = N 58°51'10" W

(172 WL)

383.79' (TOTAL)
181.07'

CONST. LIMITS

+00
IAPA

R = 5579.58' ARC L = 710.85' CH L = 710.37' CH B = N 46°50'49" W

N 85°47'05" W
218.32'

(176 WL)

CONST. LIMITS

R = 5579.58' ARC L = 493.32' CH L = 493.16' CH B = N 40°39'51" W

LA R/W

+50
IAPA

30.39'

312+61.51
150.00' RT.

319+91.48
150.00' RT.

R = 5579.58'
ARC L = 376.39'
CH L = 376.32'
CH B = N 52°25'45" W

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

12"TILE

CULTIVATED

N 04°55'14" E

V.M.S. 1342
V.M.S. 1278

446.49'

CULTIVATED

CURVE DATA

P.I.STA. 310+76.19
P.C.STA. 295+90.26
P.T.STA. 324+98.06
Δ = 29° 04' 41" RT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'

(175)

JAMES K. HARRIS AND
IRENE CREAMER HARRIS

DB 102 PG 449
FIRST TRACT
152.25 Ac.
030-003-0-00-010-00

PASTURE

LANDLOCKED AREA
2.983 Ac.

S 85°47'05" E
558.04'

(176) THIRD TRACT

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.

DB 160 PG 978

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.

DB 160 PG 980

FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030-003-0-00-006-00

CULTIVATED

LEGEND

● IRON PIN SET

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

CULTIVATED

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

ITEM 607 FENCE TYPE 47 = 2752 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

Q:\3590\MLRW14

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

176

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 978
GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 980
FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030–003–0–00–006–00
CULTIVATED

THIRD TRACT

FIRST TRACT

TRACT LINE 101.66 Ac.
TRACT LINE 98.59 Ac.

R = 5879.58'
ARC L = 202.62'
CH L = 202.61'
CH B = S 39°07'06" E

324+98.06
150.00' LT.

329+00.00
150.00' LT.

LA R/W — S 38°07'52" E — 401.94'

S 40°02'25" E — 1500.83' — LA R/W

CONST. LIMITS

+00 IAPA

+50 IAPA

176 WL

15"

SEE SHEET 26
MATCH LINE STA. 324 + 00

MATCH LINE STA. 338 + 00
SEE SHEET 28

325 326 327 328 329 330 331 332 333 334 335 336 337

℄ R/W, SURVEY & CONST.

S 38° 07' 52" E

CONST. LIMITS

+00 IAPA

LA R/W — N 38°07'52" W — 501.94'

+50 IAPA

CONST. LIMITS

LA R/W — N 36°20'28" W — 800.39' — LA R/W

324+98.06
150.00' RT.

330+00.00
150.00' RT.

338+00.00
175.00' RT.

P.T. STA. 324+98.06

R = 5579.58'
ARC L = 493.32'
CH L = 493.16'
CH B = N 40°39'51" W

CURVE DATA

P.I. STA. 310+76.19
P.C STA. 295+90.26
P.T. STA. 324+98.06
Δ = 29° 04' 41" RT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1485.93'
L = 2907.80'
E = 189.55'

176

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 978
GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 980
FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030–003–0–00–006–00
CULTIVATED

THIRD TRACT

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

LEGEND

● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2801 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01–08–99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGH OF WAY PLAN PROP. U.S.R. 35
STA. 324 + 00 TO STA. 338 + 00

FAY–35–2.57

27 / 40

509 / 522

Q:\3590\MLRW15

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

176

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 978
GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 980
FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030-003-0-00-006-00
CULTIVATED

FR.PORT

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 978
GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 980
FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030-003-0-00-006-00
CULTIVATED

FIRST TRACT

2 STY FR.RES.
4"METAL PIPE
EX.R/W
C.R.85
87+67.80 C.R. 85
344+00.00 200.00' LT.
350+00.00 200.00' LT.
S 40°02'25" E 1500.83' (TOTAL) 1428.97'
S 39°33'51" E 585.27'
S 41°50'37" E
LA R/W
CONST. LIMITS
STA. 341+47.44 PROP. U.S.R. 35 = STA. 85+00.00 C.R. 85
+91.3 CPA
+61.2 CPA
+98.6 CPA
+00 IAPA
484.13'
87
88
86
84
85
82
36"
15"
8"TILE
10"TILE
℄ R/W, SURVEY & CONST.
P.C. STA. 345+83.02
176 WL
339 340 341 342 343 344 345 346 347 348 349 350 351
S 38° 07' 52" E
SEE SHEET 27 MATCH LINE STA. 338 + 00
MATCH LINE STA. 352 + 00 SEE SHEET 29
+99 CPA
+10.7 CPA
+55.3 CPA
+23.1 CPA
21°
345+83.02 175.00' RT.
R = 5904.58' ARC L = 851.11' CH L = 850.37' CH B = N 42° 15' 38" W
595.64' N 38°07'52" W 783.02' (TOTAL)
338+00.00 175.00' RT.
82+62.84 C.R. 85
S 85° 41' 02" E
50' R/W MILLEDGEVILLE - OCTA RD.
36' R/W
T.R. 86 FITCHTHORN RD.

176

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 978
GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.
DB 160 PG 980
FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030-003-0-00-006-00
CULTIVATED

FIRST TRACT

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

CURVE DATA
P.I.STA. 363+90.54
P.C.STA. 345+83.02
P.T.STA. 380+84.82
Δ = 35° 01' 05" LT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 3307 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 338 + 00 TO STA. 352 + 00

FAY-35-2.57

28/40

510/522

Q:\3590\MLRW16

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

(176)

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE MARGARET COX TRUST, DATED FEBRUARY 9, 1987.

DB 160 PG 978

GLENN COX, NORMAN COX, AND VIRGIL COX AS TRUSTEES OF THE HAROLD COX TRUST, DATED FEBRUARY 9, 1987.

DB 160 PG 980

FIRST TRACT – 101.66 Ac.
THIRD TRACT – 98.59 Ac.
030–003–0–00–006–00

CULTIVATED

FIRST TRACT

DAVID L. BAUGHN AND
ALFRED HAGLER, TRUSTEES

I.P. FOUND

LANDLOCKED AREA
9.123 ACRES

(177)

FONDA GAY FICHTHORN

DB 182 PG 27
PARCEL I – SECOND TRACT
156.00 Ac.
030–006–0–00–003–00

CULTIVATED

(177 E)

S 85°57'47" E 1163.14'

S 41°50'37" E LA R/W 484.13'

355+00.00
175.00' LT.

358+68.51
175.00' LT.

R = 5554.58' ARC L = 357.25' CH L = 357.19' CH B = S 49°08'37" E

R = 5554.58' ARC L = 1324.95' CH L = 1321.81' CH B = S 57°49'10" E LA R/W

CONST. LIMITS

+00 IAPA

+50 IAPA

(176 WL)

(177 WL)

577.43' (TOTAL)
296.30

SEE SHEET 28
MATCH LINE STA. 352 + 00

MATCH LINE STA. 366 + 00
SEE SHEET 30

15"

18"

12"TILE

353 354 355 356 357 358 ℄ 359 360 361 362 363 364 365

R/W, SURVEY & CONST.

356+25.65 ℄

10"TILE

R = 5904.58'
ARC L = 851.11'
CH L = 850.37'
CH B = N 42°15'38" W

+00 IAPA

354+08.90
175.00' RT

N 85°57'47" W 281.13'

LA R/W

CONST. LIMITS

R = 5904.58' ARC L = 1433.59' CH L = 1430.07' CH B = N 53°20'44" W LA R/W

+00 IAPA

+00 IAPA

127.99'

10"TILE

12"TILE

SECOND TRACT

CURVE DATA

P.I. STA. 363+90.54
P.C. STA. 345+83.02
P.T. STA. 380+84.82
Δ = 35° 01' 05" LT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'

(177)

FONDA GAY FICHTHORN

DB 182 PG 27
PARCEL I – SECOND TRACT
156.00 Ac.
030–006–0–00–003–00

CULTIVATED

LEGEND

● IRON PIN SET

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2800 L.F.
CARRIED TO SHEET NO.42

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 177 E

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 352 + 00 TO STA. 366 + 00

FAY–35–2.57

29/40

511/522

Q:\3590\MLRW17

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

DAVID L. BAUGHN AND
ALFRED HAGLER, TRUSTEES

I.P. FOUND

369+71.85
749.22 LT.

S 85°57'47" E
1163.14'

LANDLOCKED AREA
9.123 ACRES

623.48'
S 03°49'01" W

(177)

FONDA GAY FICHTHORN

DB 182 PG 27
PARCEL I – SECOND TRACT
156.00 Ac.
030–006–0–00–003–00

CULTIVATED

(177 E)

(178)

THEODORE D. CRAIG
AND DAVID W. CRAIG

DB 145 PG 649
DB 150 PG 785
78.97 Ac.
030–003–0–00–003–00

CULTIVATED

CURVE DATA

P.I. STA. 363+90.54
P.C. STA. 345+83.02
P.T. STA. 380+84.82
Δ = 35° 01' 05" LT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'

+00
IAPA

LA R/W

R = 5554.58' ARC L = 1324.95' CH L = 1321.81' CH B = S 57° 49' 10" E

CONST. LIMITS

372+35.20
175.00' LT.

+50
IAPA

R = 5554.58' ARC L = 823.67' CH L = 822.92' CH B = S 68° 54' 04" E

LA R/W

CONST. LIMITS

347.89'
187.68' (TOTAL)

(177 WL)

(178 WL)

MATCH LINE STA. 366 + 00
SEE SHEET 29

MATCH LINE STA. 380 + 00
SEE SHEET 31

54"

18"

12" TILE

367 368 369 370 371 372 373 374 375 376 377 378 379

373+04.08 ℄

℄ R/W, SURVEY & CONST.

R = 5904.58'
ARC L = 1433.59'
CH L = 1430.07'
CH B = N 53°20'44" W

+00
IAPA

+00
IAPA

105.83'
N 74°27'52" W

LA R/W

CONST. LIMITS

R = 5879.58' ARC L = 471.66' CH L = 471.53' CH B = N 63° 35' 57" W

160.21'

R = 5879.58' ARC L = 744.18' CH L = 743.69' CH B = N 69° 31' 23" W

+00
IAPA

CONST. LIMITS

LA R/W

368+00.00
175.00' RT.

373+59.63
150.00' RT.

+50
IAPA

(178)

THEODORE D. CRAIG
AND DAVID W. CRAIG

DB 145 PG 649
DB 150 PG 785
78.97 Ac.
030–003–0–00–003–00

CULTIVATED

LANDLOCKED AREA
2.837 ACRES

170.43'

(179)

McDONALDS FARMS, INC.

DB 141 PG 384
70.75 Ac.
030–006–0–00–004–00

TO BE ACQUIRED UNDER
FAY–35–7.51

CULTIVATED

(177)

FONDA GAY FICHTHORN

DB 182 PG 27
PARCEL I – SECOND TRACT
156.00 Ac.
030–006–0–00–003–00

CULTIVATED

LEGEND

- ● IRON PIN SET
- ○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

FENCE LEGEND

EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2798 L.F.
CARRIED TO SHEET NO.42

I.P. (Fnd)

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

REV.	DATE	DESCRIPTION
WGY	12-27-99	ADDED PARCEL 177 E

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 366 + 00 TO STA. 380 + 00

FAY–35–2.57

30 / 40

512 / 522

Q:\3590\MLRW18

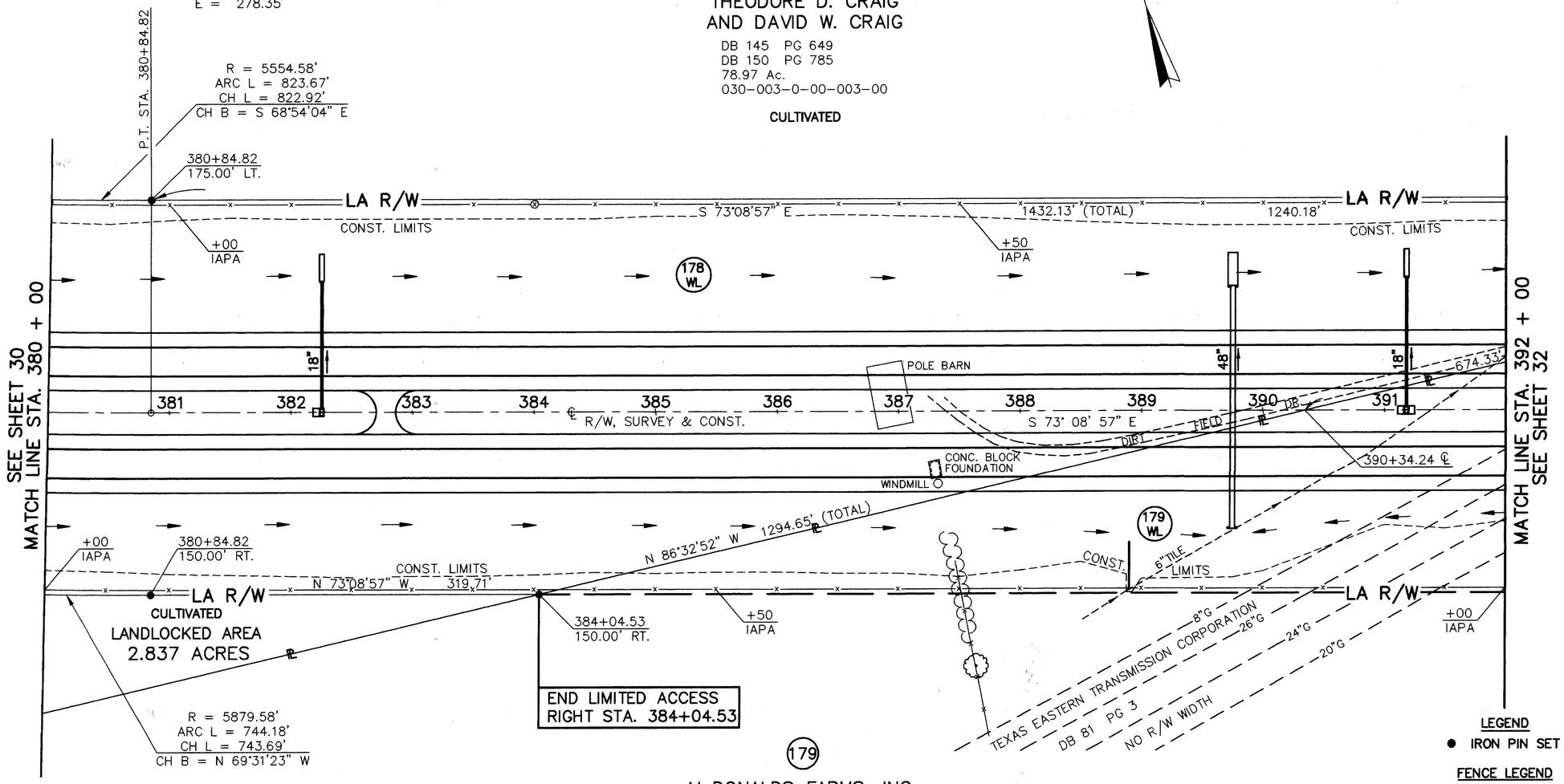
FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

CURVE DATA
P.I. STA. 363+90.54
P.C. STA. 345+83.02
P.T. STA. 380+84.82
Δ = 35° 01' 05" LT.
Dc= 1° 00' 00"
R = 5729.58'
T = 1807.52'
L = 3501.80'
E = 278.35'

178

THEODORE D. CRAIG
AND DAVID W. CRAIG
DB 145 PG 649
DB 150 PG 785
78.97 Ac.
030-003-0-00-003-00

CULTIVATED



179

McDONALDS FARMS, INC.
DB 141 PG 384
70.75 Ac.
030-006-0-00-004-00

TO BE ACQUIRED UNDER
FAY-35-7.51

CULTIVATED

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 2400 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 380 + 00 TO STA. 392 + 00

FAY-35-2.57

31 / 40

513 / 522

Q:\3590\MLRW19

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

398+79.00 (BACK STA.)
692.75' LT.

S 10°01'50" W
20.75'

398+81.46 (BACK STA.)
672.14' LT.

S 04°39'44" W
89.47'

399+00.35 (BACK STA.)
584.69' LT.

HARMONY RD.

CHANNEL EASEMENT
N 60°27'48" E 422.36'

CHANNEL EASEMENT
S 60°27'48" W 358.85'

178

THEODORE D. CRAIG &
DAVID W. CRAIG
DB 145 PG 649
DB 150 PG 785
78.97 Ac.
030-003-0-00-003-00

CULTIVATED

395+87.66
386.95' LT.

178 X

396+52.83 (BACK STA.)
324.87' LT.

CHANNEL CURVE DATA
P.I. STA. 6+91.48
P.C. STA. 5+01.64
P.T. STA. 8+60.93
Δ = 45° 44' 45" LT.
Dc= 12° 43' 57"
R = 450.00'
T = 189.84'
L = 359.29'
E = 38.40'

337.51'
N 67°56'59" E
P.T.
CONST. LIMITS
202.30'
S 59°02'49" W

8"G
26"G
24"G
20"G
TEXAS EASTERN TRANSMISSION CORPORATION

EX.R/W
EX.R/W
EX.R/W

393+25.00
175.00' LT.
+50
EPA

1240.18'
LA R/W
S 73°08'57" E 1432.13' (TOTAL)
191.95'

+75
EPA
395+16.95
175.00' LT.

S 16°51'03" W
25.00'

DIRT FIELD DR.
LA R/W

+15, CPA
S 73°08'57" E 147.00'

395+16.95
150.00' LT.

396+63.95
EPA

396+63.95 (BACK STA.)
150.00' LT.

END LIMITED ACCESS
LEFT STA. 396+63.95 (BACK STA.)

178 WL
P.T.
15"TILE
N 86°32'52" W 1294.65'(TOTAL)
CONST. LIMITS

LA R/W

C.R.74

SEE SHEET 31
MATCH LINE STA. 392+00

TEXAS EASTERN TRANSMISSION CORP.
DB 81 PG 3
NO R/W WIDTH

393 394 395 396 381 382 383 384 385 386
℄ R/W, SURVEY & CONST.
S 73° 08' 57" E

CONST. LIMITS

+00
IAPA
396+20.24
EPA

LA R/W

179

McDONALDS FARMS, INC.
DB 141 PG 384
70.75 Ac.
030-006-0-00-004-00

TO BE ACQUIRED UNDER
FAY-35-7.51

END PROJECT
FAY-35-2.57
STA. 396+20.24 BACK =
STA. 380+00.00 AHEAD

END WORK
FAY-35-2.57
STA. 396+44.86
(BACK STATION)

END R/W ACQUISITION
FAY-35-2.57
STA. 399+00.35
(BACK STATION)

LEGEND
● IRON PIN SET

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 784 L.F.
CARRIED TO SHEET NO.42

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1342

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN PROP. U.S.R. 35
STA. 392 + 00 TO STA. 386 + 00

FAY-35-2.57

32/40

514/522

Q:\3590\MLRW20

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

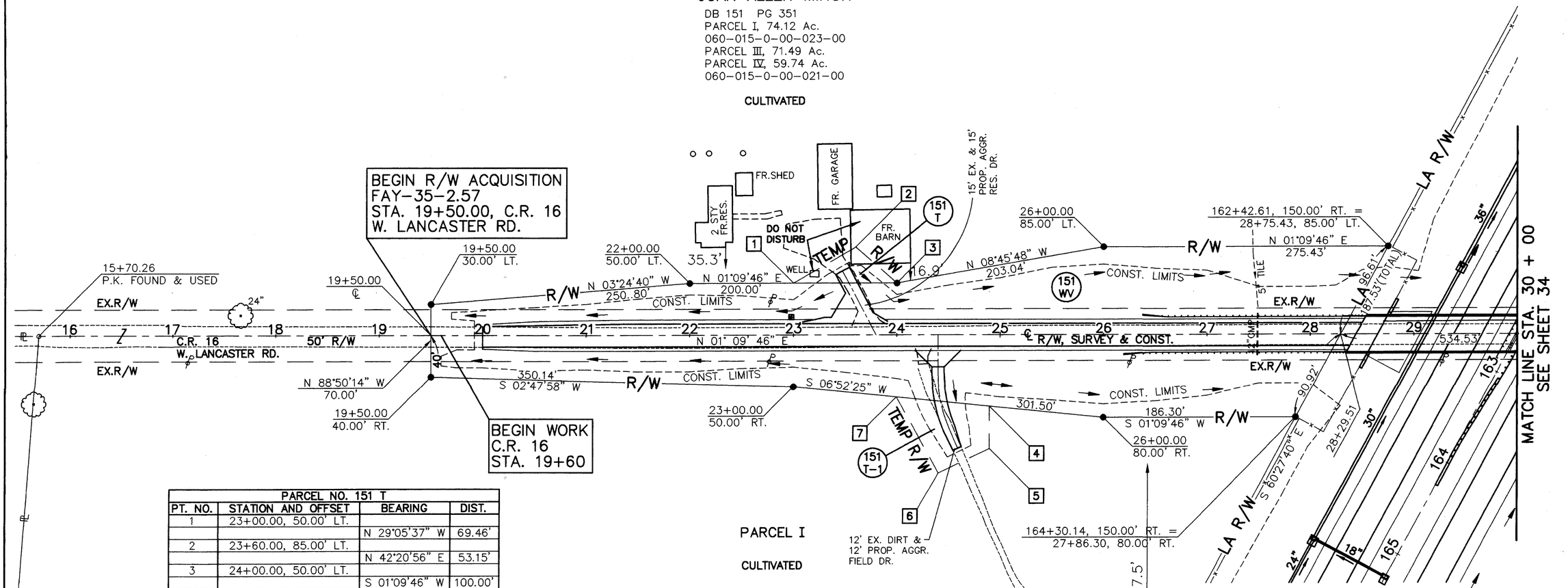
PARCEL IV

(151)

JOAN ALLEN MINOR

DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00

CULTIVATED



PARCEL NO. 151 T			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
1	23+00.00, 50.00' LT.		
		N 29°05'37" W	69.46'
2	23+60.00, 85.00' LT.		
		N 42°20'56" E	53.15'
3	24+00.00, 50.00' LT.		
		S 01°09'46" W	100.00'
1	23+00.00, 50.00' LT.		

PARCEL NO. 151 T-1			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
4	24+90.00, 69.00' RT.		
		S 88°50'14" E	41.00'
5	24+90.00, 110.00' RT.		
		S 25°24'08" E	55.90'
6	24+40.00, 135.00' RT.		
		S 63°05'26" W	85.00'
7	24+00.00, 60.00' RT.		
		N 06°52'25" E	90.45'
4	24+90.00, 69.00' RT.		

PARCEL I

CULTIVATED

(151)

JOAN ALLEN MINOR

DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS - COUNTY RECORDS

PK FOUND - STA. 15+70.26

FOR R/W FENCE LOCATION, SEE R/W SHEET 15

LEGEND

- ● IRON PIN SET
- o RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0, 50, 100

P I D NO. 9078

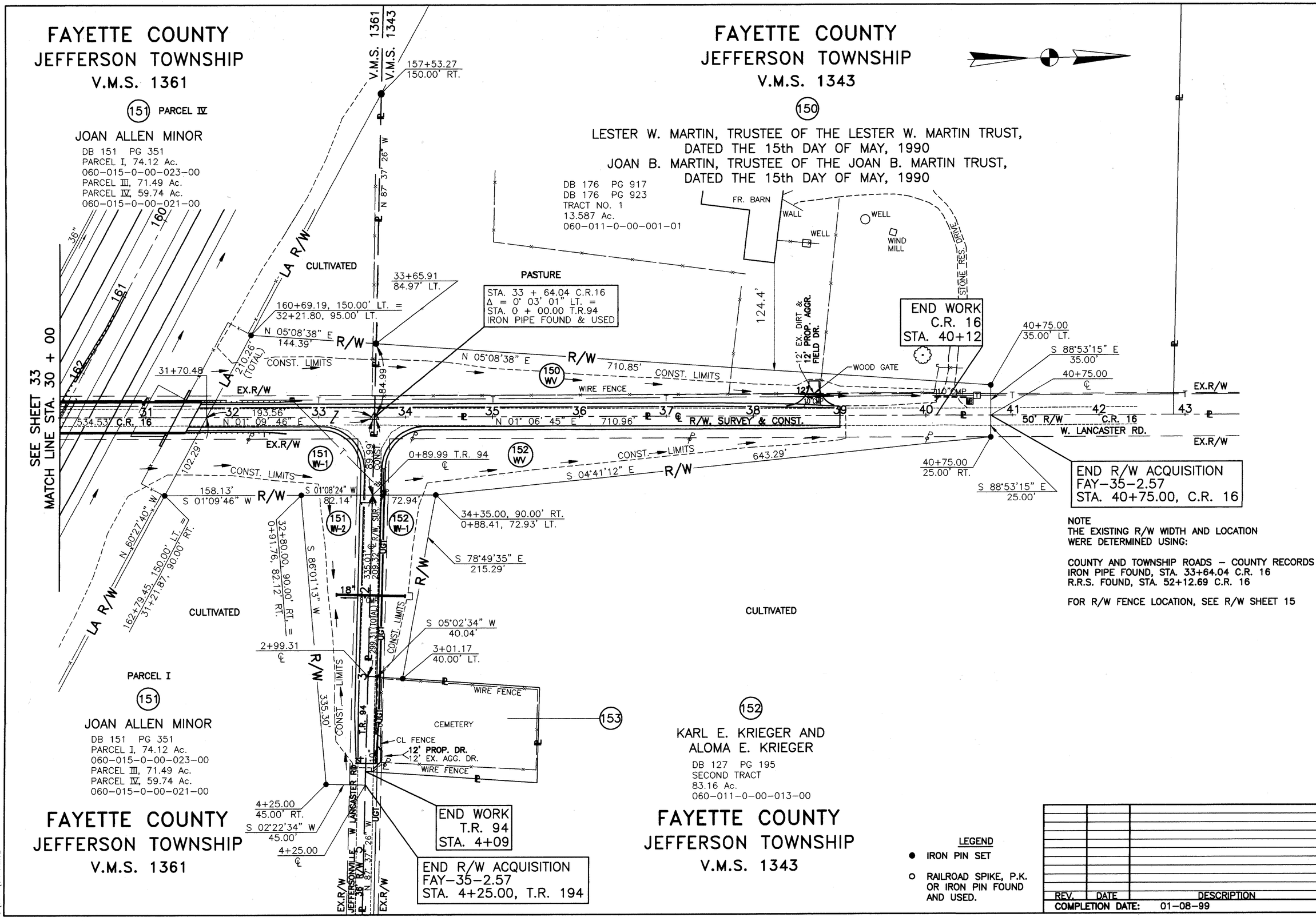
RIGHT OF WAY PLAN C.R. 16
STA. 16 + 00 TO STA. 30 + 00

FAY-35-2.57

33 / 40

515
522

Q:\3590\CR16RW01


FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361
151 PARCEL IV
JOAN ALLEN MINOR
DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343
150
LESTER W. MARTIN, TRUSTEE OF THE LESTER W. MARTIN TRUST,
DATED THE 15th DAY OF MAY, 1990
JOAN B. MARTIN, TRUSTEE OF THE JOAN B. MARTIN TRUST,
DATED THE 15th DAY OF MAY, 1990
DB 176 PG 917
DB 176 PG 923
TRACT NO. 1
13.587 Ac.
060-011-0-00-001-01
FR. BARN
WALL
WELL
WIND MILL
STONE RES. DRIVE
V.M.S. 1361
V.M.S. 1343
157+53.27
150.00' RT.
N 87° 37' 26" W
CULTIVATED
PASTURE
33+65.91
84.97' LT.
160+69.19, 150.00' LT. =
32+21.80, 95.00' LT.
N 05°08'38" E
144.39'
R/W
STA. 33 + 64.04 C.R.16
Δ = 0° 03' 01" LT. =
STA. 0 + 00.00 T.R.94
IRON PIPE FOUND & USED
124.4'
12' EX. DIRT &
12' PROP. AGGR.
FIELD DR.
END WORK
C.R. 16
STA. 40+12
40+75.00
35.00' LT.
S 88°53'15" E
35.00'
40+75.00
℄
WOOD GATE
N 05°08'38" E
710.85'
CONST. LIMITS
31+70.48
210.26' (TOTAL)
LA R/W
EX.R/W
WIRE FENCE
150 WV
SEE SHEET 33
MATCH LINE STA. 30 + 00
534.53' C.R. 16
193.56'
N 01° 09' 46" E
N 01° 06' 45" E
710.96'
R/W, SURVEY & CONST.
50' R/W
C.R. 16
W. LANCASTER RD.
EX.R/W
151 WV-1
152 WV
0+89.99 T.R. 94
℄
CONST. LIMITS
643.29'
S 04°41'12" E
R/W
40+75.00
25.00' RT.
S 88°53'15" E
25.00'
END R/W ACQUISITION
FAY-35-2.57
STA. 40+75.00, C.R. 16
158.13'
S 01°09'46" W
S 01°08'24" W
82.14'
72.94'
34+35.00, 90.00' RT.
0+88.41, 72.93' LT.
151 WV-2
152 WV-1
S 78°49'35" E
215.29'
N 60°27'40" W
162+79.45, 150.00' LT. =
31+21.87, 90.00' RT.
32+80.00, 90.00' RT. =
0+91.76, 82.12' RT.
S 86°01'13" W
CULTIVATED
CULTIVATED
S 05°02'34" W
40.04'
3+01.17
40.00' LT.
2+99.31
℄
PARCEL I
151
JOAN ALLEN MINOR
DB 151 PG 351
PARCEL I, 74.12 Ac.
060-015-0-00-023-00
PARCEL III, 71.49 Ac.
PARCEL IV, 59.74 Ac.
060-015-0-00-021-00
335.30'
CONST. LIMITS
T.R. 94
WIRE FENCE
CEMETERY
153
CL FENCE
12' PROP. DR.
12' EX. AGG. DR.
WIRE FENCE
4+25.00
45.00' RT.
S 02°22'34" W
45.00'
4+25.00
℄
W LANCASTER RD
END WORK
T.R. 94
STA. 4+09
END R/W ACQUISITION
FAY-35-2.57
STA. 4+25.00, T.R. 194
EX.R/W
JEFFERSONVILLE
36" R/W
N 87° 37' 26" W
EX.R/W
152
KARL E. KRIEGER AND
ALOMA E. KRIEGER
DB 127 PG 195
SECOND TRACT
83.16 Ac.
060-011-0-00-013-00
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361
FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1343
LEGEND
● IRON PIN SET
○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.
REV. DATE DESCRIPTION
COMPLETION DATE: 01-08-99
SCALE IN FEET
0 50 100
P I D NO. 9078
RIGHT OF WAY PLAN C.R. 16 & T.R. 94
STA. 30+00 TO STA. 43+00 C.R. 16
STA. 0+00 TO STA. 5+00 T.R. 94
FAY-35-2.57
34/40
516/522
Q:\3590\CR16RW02

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY RECORDS
S.H. 29 – SEC. Gpt & Hpt
FAY–35–3.31
FAY–35 & FACTORY SHOP BOULVARD

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1361

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

(158) THE LYDEN COMPANY
AN OHIO CORPORATION

(159) CROWN PROPERTY DEVELOPMENT,
AN OHIO CORPORATION
DB 168 PG 93
5.299 Ac.
060–015–0–00–037–00

(154) COLORADO LAND COMPANY, LTD.
VOL 50 PG 889
VOL 50 PG 894
18.355 ACRES
060–016–0–00–004–01

PARCEL NO. 159 WL			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
10	66+40.00, 12.00' RT.		
		N 30°37'17" E	96.10'
14	66+40.00, 84.10' LT.		
		S 03°09'41" W	108.31'
11	66+89.94, 12.00' RT.		
		N 59°22'43" W	49.94'
10	66+40.00, 12.00' RT.		

P I D NO. 9078

SCALE IN FEET 0 50 100

P.C. STA. 94+51.0345 TEMP. RD.1=
STA. 58+87.8791 EX. USR 35

61+99.47, 125.00' LT. = (62+00.00FAY–1–2.35)
61+86.83 125.00' LT. I.P. (Fnd)
BEGIN R/W ACQUISITION
FAY–35–2.57
STA.61+99.47,EX.U.S.R.35
EX.R/W
32' EX. & 32' PROP. ASPH. DR.
P.R.C.
S 59° 22' 43" E 60
R/W, SURVEY & CONST. (FAY–1–2.35)
95
UNKNOWN OWNERSHIP
PK (Fnd)
61+99.47, 12.00' RT. = (62+00.00 FAY–1–2.35)
UGT
BROKEN WOOD POSTS
CONC. MON. FOUND
N 30°37'17" E 63.00'
S 30°38'09" W
N 30°37'17" E 137.00'
107.68'
63+07.13 12.00' RT.
213.32'
18" CONC.
22"x34" TWIN
119.53'
63+07.13 75.00' RT.
61+99.47, 75.00' RT. = (62+00.00FAY–1–2.35)
N 59°22'43" W 107.66'
EX.R/W
8" PLASTIC
CONST. LIMITS
217.87'
65+25.00 75.00' RT.
R/W
SERVICE RD. 1
N 13°12'47" E 330.02'
N 10°07'01" E 268.94'
S 36°37'17" W 70.39'
EX. M.H.
8"SAN.
S 70°49'26" E TEMP. R/W 352.76'
TEMP. CONST. LIMITS
65+37.86 125.00' LT.
S 59°22'43" E 338.39'
I.P. (Fnd)
100
CONST. LIMITS
6" PLASTIC
12" CONC.
COUNTY OF FAYETTE
DB 173 PG 1073
65+20.47 12.00' RT.
65+51.47 254.46' LT.
65+45.22 195.00' LT.
COMMERCIAL
TANK
WALL
S 53°22'43" E 100.54'
S 03°09'41" W
V.M.S. 1223
V.M.S. 1361
66+51.45 243.96' LT.
66+48.41 215.00' LT.
8"SAN.
EX. M.H.
154 WD
109.56'
80.44'
N 36°37'17" E 130.18'
191.99'
S 36°37'17" W
CULTIVATED
73+00.00 215.00' LT.
TEMP. R/W
S 59°22'43" E 651.59'
66+40.00 135.00' LT.
+42 EPA
TEMP. CONST. LIMITS
105
P.T.
℄ TEMP. ROAD
LA R/W
N 30°37'17" E 50.90'
S 59°22'43" E 792.18'
CONST. LIMITS
+00 IAPA
108.37'
14
159 WL
154 WL
24"
12" CONC
71+60.39 12.00' RT.
EX.R/W
10
11
R.R.S. (Fnd)
67
68
N 59° 22' 43" W 470.45'
856.26'
69
70
℄ R/W, SURVEY & CONST.
226
71
72
252.21'
227
228
UGT
RAMP C
MATCH LINE STA. 73 + 00
SEE SHEET 20
RIGHT OF WAY PLAN EX.U.S.R. 35
STA. 60 + 00 TO STA. 73 + 00
FAY–35–2.57
35 / 40
517 / 522

LEGEND
- ● IRON PIN SET
- ○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.
- THE OHIO BELL TELEPHONE COMPANY EASEMENT DB 175 PG 856

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

TERRY GARNER
OR 34 PG 749
2.430 Ac.
060–015–0–00–043–02

MICHAEL J. GARNER
OR 41 PG 692
2.430 Ac.
060–015–0–00–043–01

BEGIN WORK STA. 62+25

DAVID L. GARNER
OR 37 PG 1019
1.215 Ac.(Deed) 1.226 Ac.(Calc)
060–015–0–00–043–03

CARL H. JAMES AND BERNICE I. JAMES
DB 170 PG 423
171.522 Ac.

DB 98 PG 515
DB 103 PG 709
TRACT NO. 2
2.31 Ac.
060–015–0–00–042–00

LEENORA MORROW
LEENORA B. MORROW

TRACT NO. 1
2.18 Ac.
060–015–0–00–041–00

+42 EPA
+75 IAPA
+30 IAPA
+25.34'
SAVE
12" PROP. AGG. FIELD DR.
8" EX. RES. DR.
12" PROP. AGG. RES. DR.
TEMP. R/W
179.3'
2 STY FR.RES.
TANK
BLOCK GARAGE
66+19.20 326.90' RT.
N 84°43'51" W 210.40'
68+09.35 236.81' RT.
S 02°38'51" W
N 04°16'51" E 125.75'
72+16.19 124.69' RT.
460.78'
N 50°00'39" W
N 02°38'51" E 28.66'
1147.91'
N 30°25'28" W
72+29.63 150.00' RT.
RESIDENTIAL 165

PARCEL NO. 165 T			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
1	68+00.00, 79.12' RT.		
		S 50°00'39" E	50.68'
2	68+50.00, 87.37' RT.		
		S 17°45'52" W	89.88'
3	68+70.00, 175.00' RT.		
		S 03°39'56" E	133.14'
4	69+45.00, 285.00' RT.		
		S 86°55'53" W	54.08'
5	69+00.00, 315.00' RT.		
		N 06°14'54" W	100.00'
6	68+40.00, 235.00' RT.		
		N 62°45'24" W	30.71'
7	68+09.35, 236.81' RT.		
		N 02°38'51" E	98.29'
8	67+63.24, 150.00' RT.		
		S 59°22'43" E	36.76'
9	68+00.00, 150.00' RT.		
		N 30°37'17" E	70.88'
1	68+00.00, 79.12' RT.		

PARCEL NO. 164 WD			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
10	66+40.00, 12.00' RT.		
		S 30°37'17" W	63.00'
13	66+40.00, 75.00. RT.		
		S 59°22'43" E	83.40'
12	67+23.40, 75.00' RT.		
PARCEL NO. 164 WL			
10	66+40.00, 12.00' RT.		
		S 59°22'43" E	49.94'
11	66+89.94, 12.00' RT.		
		S 02°38'51" W	71.34'
12	67+23.40, 75.00' RT.		
		N 59°22'43" W	83.40'
13	66+40.00, 75.00' RT.		
		N 30°37'17" E	63.00'
10	66+40.00, 12.00' RT.		

PARCEL NO. 165 WL			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
15	67+75.00, 75.00' RT.		
		N 59°22'43" W	51.60'
12	67+23.40, 75.00' RT.		
		N 02°38'51" E	71.34'
11	66+89.94, 12.00' RT.		

ITEM 607 FENCE TYPE 47 = 1338 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	11–1–99	ADDED PARCEL 154
WGY	9–30–99	REVISED PARCELS 162, 163, AND 164

COMPLETION DATE: 01–08–99

Q:\3590\EX35RW01

FAYETTE COUNTY
JEFFERSON TOWNSHIP
V.M.S. 1223

CURVE DATA
P.I. STA. 81+85.44
P.C. STA. 80+00.00
P.R.C. STA. 83+70.84
Δ = 01° 51' 15" RT.
Dc= 0° 30' 00"
R = 11,459.16'
T = 185.44'
L = 370.84'
E = 6.00'

CURVE DATA
P.I. STA. 85+56.28
P.R.C. STA. 83+70.84
P.T. STA. 87+41.68
Δ = 01° 51' 15" LT.
Dc= 0° 30' 00"
R = 11,459.16'
T = 185.44'
L = 370.84'
E = 6.00'

(161)

STEVEN L. SOLLARS & LISA E. SOLLARS,
BRYAN H. SOLLARS & MARY K. SOLLARS,
MICHAEL D. SOLLARS & SHERRY A. SOLLARS,
JEFFERY A. SOLLARS & BRIDGET M. SOLLARS.

DB 182 PG 347
DB 182 PG 509
FIRST TRACT – 54.42 ACRES
060-016-0-00-008-00
SECOND TRACT – 15.51 ACRES
060-016-0-00-007-01
FOURTH TRACT – 35.97 ACRES
060-016-0-00-009-00

50' EASEMENT DB 117 PG 320 THE DAYTON POWER & LIGHT COMPANY

35' EASEMENT DB 184 PG 215 BOARD OF FAYETTE COUNTY COMMISSIONERS

SECOND TRACT
CULTIVATED

FIRST TRACT
CULTIVATED

82+00.00 251.78' LT.
8"SAN.
EX. M.H.
R/W S 59°22'43" E 1195.49'
94+00.00 262.00' LT.
CONST. LIMITS
24"x38"
161 WD
24"
182.00' S 30°37'17" W
94+00.00 170.00' LT.
18 19 20 21 22 23 SERVICE RD. NO. 3 24 25 26 27 28
℄ R/W, SURVEY & CONST. S 59° 22' 43" E
1075.17'
LA S 59°22'43" E
TEMP. R/W
S 46°58'16" E 511.96'
161 T
TEMP. CONST. LIMITS
+00 IAPA
STA. 84+29.06 EX. U.S.R. 35 = STA. D 236+98.70
P.R.C.
161 WL
+50 IAPA
P.C.
14' EX. AGGR. FIELD DR.
TANK
WELL
WIND MILL
STY FR. RES.
FLOWER BED w/R.R. TIES
92+00.00
92+00.00 132.00' LT.
+98 EPA
182.00' N 30°37'17" E 132.00'
15"
P.T.
94+00.00 80.00' LT.
130 N 57°05'17" W
R/W 500.40'
CONST. LIMITS
EX.R/W
705.81'
83 84 85 86 705.81 87 88 89 90 91 92 93 R/W 94 95 1202.39'
S 59° 22' 43" E 1738.42'
℄ R/W, SURVEY & CONST.
EX. CB
12" CONC.
6" TILE (ABANDON)
24"
P.T.
6" TILE
UGT
12" CMP
88+98.95
301.05'
S 59° 22' 43" E
12' PROP. AGGR. FIELD DR.
EX.R/W
30"
+72 CPA
CONST. LIMITS
4' CONC. PILLAR
265.23' N 63°49'51" W
+98 EPA
LA R/W
363.12' N 63°49'51" W
24' EX. DIRT FIELD DR.
EX. DIRT FIELD DR.
89+35.57 70.59' RT.
92+00.00 50.00' RT.
P.C.
RAMP D
R=30
CONST. LIMITS
85+75.00 100.00' RT.
LA R/W
S 09°50'41" W 316.52'
N 03°11'54" E
25' EASEMENT DB 117 PG 320
25' EASEMENT DB 117 PG 322
167 WL
CULTIVATED
LANDLOCKED AREA
5.867 ACRES
(167)
LARRY L. MAXEL AND
VIRGINIA M. MAXEL
DB 142 PG 398
162.967 Ac.
030-002-0-00-017-00
30.339 Ac.
060-015-0-00-038-00
DB 162 PG 598
FIRST TRACT – 26.93 Ac.
060-015-0-00-039-00
SECOND TRACT – 0.320 Ac.
060-015-0-00-040-00
167 E
50' EASEMENT DB 117 PG 322 THE DAYTON POWER & LIGHT COMPANY
S 03°11'54" W 752.39'
SEE SHEET 21
MATCH LINE STA. 240 + 00 SEE SHEET 21
MATCH LINE STA. 82 + 00 SEE SHEET 20
MATCH LINE STA. 96+00 SEE SHEET 37

(161)

STEVEN L. SOLLARS & LISA E. SOLLARS,
BRYAN H. SOLLARS & MARY K. SOLLARS,
MICHAEL D. SOLLARS & SHERRY A. SOLLARS,
JEFFERY A. SOLLARS & BRIDGET M. SOLLARS.

DB 182 PG 347
DB 182 PG 509
FIRST TRACT – 54.42 ACRES
060-016-0-00-008-00
SECOND TRACT – 15.51 ACRES
060-016-0-00-007-01
FOURTH TRACT – 35.97 ACRES
060-016-0-00-009-00

CULTIVATED

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY RECORDS
S.H. 29 – SEC. Gpt & Hpt
FAY-35-3.31

LEGEND
● IRON PIN SET
○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

FENCE LEGEND
EPA = END POST ASSEMBLY
IAPA = INTERMEDIATE ANCHOR POST ASSEMBLY
CPA = CORNER POST ASSEMBLY

ITEM 607 FENCE TYPE 47 = 1771 L.F.
CARRIED TO SHEET NO.42

REV.	DATE	DESCRIPTION
WGY	11-1-99	ADDED PARCEL 167E

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

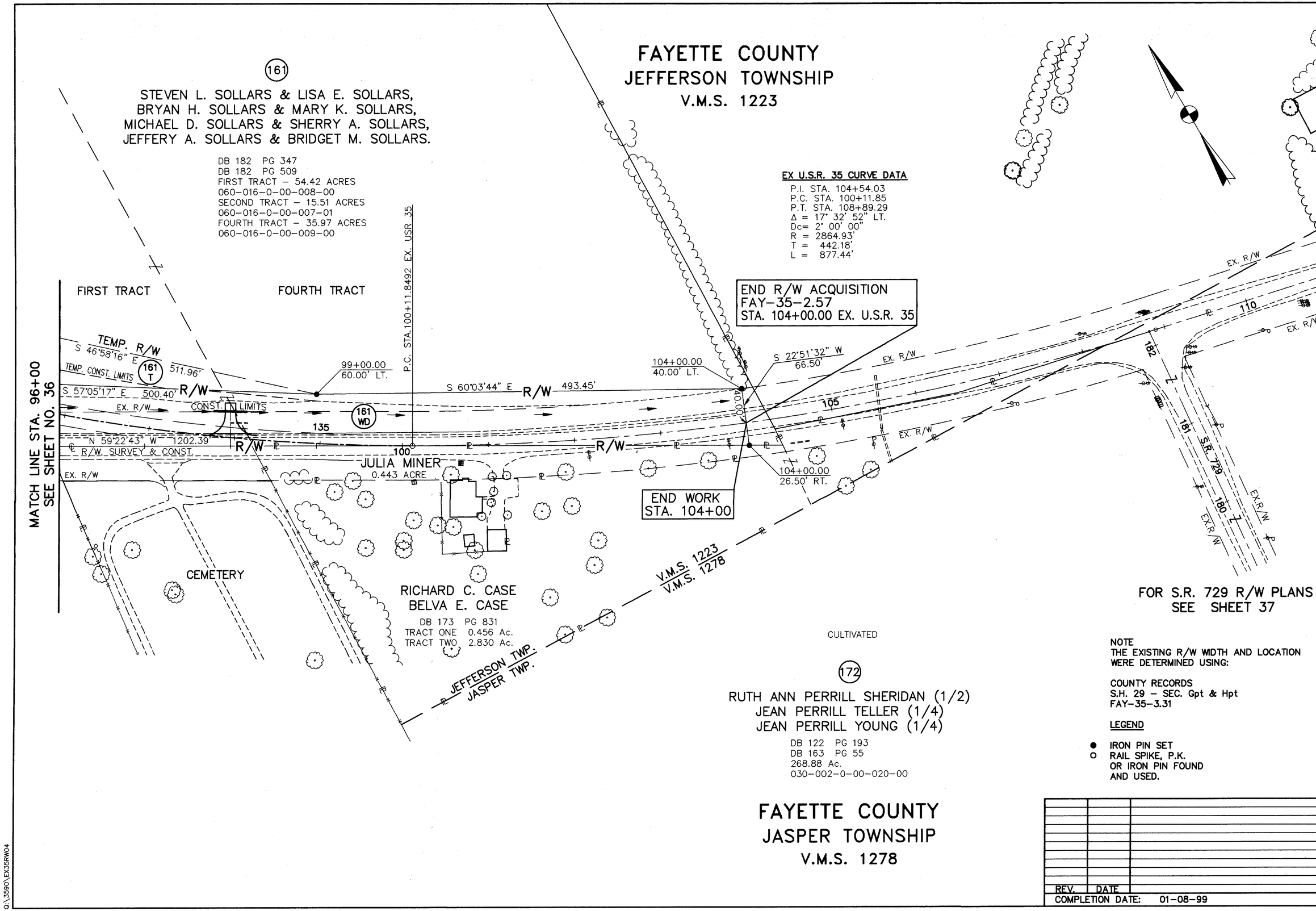
RIGHT OF WAY PLAN EX.U.S.R. 35
STA. 86 + 00 TO STA. 96 + 00

FAY-35-2.57

36 / 40

518 / 522

Q:\3590\EX35RW01



NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY RECORDS
S.H. 29 – SEC. Gpt & Hpt
FAY-35-3.31

LEGEND

● IRON PIN SET
○ RAIL SPIKE, P.K. OR IRON PIN FOUND AND USED.

REV.	DATE

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN EX.U.S.R. 35
STA. 96 + 00 TO STA. 110 + 00

FAY-35-2.57

37/40

519/522

Q:\3590\EX35RW04

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

PARCEL NO. 167 T			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
8	153+26.83, 50.56' LT.		
		S 01°59'27" E	42.22'
9	152+85.00, 44.81' LT.		
		N 84°09'43" W	15.19'
10	152+85.00, 60.00' LT.		
		N 05°50'17" E	41.11'
11	153+26.11, 60.00' LT.		
		S 88°29'40" E	9.46'
8	153+26.83, 50.56' LT.		

PARCEL NO. 167 T-1			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
12	155+45.55, 80.64' LT.		
		N 88°29'40" W	59.53'
15	155+41.05, 140.00' LT.		
		N 05°50'17" E	38.95'
16	155+80.00, 140.00' LT.		
		N 74°45'16" E	55.60'
17	156+00.00, 88.13' LT.		
		S 01°59'27" E	54.96'
12	155+45.55, 80.64' LT.		

CULTIVATED

(167)

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 148 PG 431
124.59 Ac.
030-002-0-00-018-00

(171)

IVA I. PEEL AND
EUGENE L. PEEL

DB 159 PG 109
8.225 Ac.
030-002-0-00-019-00

RESIDENTIAL

(167)

LARRY L. MAXEL AND
VIRGINIA M. MAXEL

DB 148 PG 431
124.59 Ac.
030-002-0-00-018-00

CULTIVATED

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

BEGIN R/W ACQUISITION
FAY-35-2.57
STA. 152+50.00, S.R. 729

BEGIN WORK
S.R. 729
STA. 152+75

135+33.27, MON. BOX
1" I.P.(FND. & USED)

152+50.00 ℄
N 84°09'43" W
40.00'

152+50.00
40.00' RT.

12' EX. DIRT & 12' PROP. AGGR. FIELD DR.

8' EX. & 12' PROP. AGGR. RES. DR.

TEMP. R/W

TEMP R/W

RES. DR.

FIELD DR.

12' EX. DIRT & 12' PROP. AGGR. FIELD DR.

156+50.00
95.00' LT.

N 03°59'29" E
508.42'

R/W

N 01°59'27" W
220.78

105.43'
N 01°59'27" W

CONST. LIMITS

METAL ℄POST

4'CONC. PILLAR

80.87'

EX.R/W

S.R. 729
50' R/W

UGT

151 152 153 154 221.00 155 156 157 158 159 614.73 160 161 162 689.56

N 05° 50' 17" E
℄ R/W, SURVEY & CONST.
916.39'

12"CLAY

155+51.66 ℄

4'CONC. PILLAR

4" CPP EXTENDED ABOVE GROUND

S 11°32'56" W
402.00'

R/W

CONST. LIMITS

S 08°30'52" W
856.68'

MATCH LINE STA. 163 + 00
SEE SHEET 39

N 88°29'40" W

N 88°23'34" W

N 05°50'17" E
50.99'

161+49.01
235.00' LT.

162+00.00
235.00' LT.

WIRE FENCE

123.95'

SAVE

TEMP R/W

N 55°46'24" E
CONST. LIMITS
155.47'

12' FIELD DR.

161+58.16
111.39' LT.

172 T-1
141.92'

163+00.00
115.96' LT.

N 03°59'29" E 422.49'

111.69'

161+66.39 ℄

8' FIELD DR. w/WIRE GATE

10' EX. & 12' PROP. AGGR. RES. DR.

TEMP. R/W

TANK

252.8'

251.7'

2 STY FRAME BARN

FRAME PORT

5" TILE

PARCEL NO. 167 WD			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
6	152+50.00, ℄		
		N 84°09'43" W	40.00'
7	152+50.00, 40.00' LT.		
		N 01°59'27" W	77.55'
8	153+26.83, 50.56' LT.		
		S 88°29'40" E	50.71'
5	153+30.66, ℄		
		S 05°50'17" W	80.66'
6	152+50.00, ℄		

PARCEL NO. 171 T			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
12	155+45.55, 80.64' LT.		
		S 01°59'27" E	61.12'
13	154+85.00, 72.31' LT.		
		N 71°40'00" W	69.33'
14	155+00.00, 140.00' LT.		
		N 05°50'17" E	41.05'
15	155+41.05, 140.00' LT.		
		S 88°29'40" E	59.53'
12	155+45.55, 80.64' LT.		

PARCEL NO. 172 T			
PT. NO.	STATION AND OFFSET	BEARING	DIST.
1	156+50.00, 80.00' RT.		
		S 62°21'38" E	26.93'
2	156+40.00, 105.00' RT.		
		S 01°17'13" E	40.31'
3	156+00.00, 110.00' RT.		
		S 67°26'43" W	42.06'
4	155+80.00, 73.00' RT.		
		N 11°32'56" E	70.35'
1	156+50.00, 80.00' RT.		

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS

LEGEND
- ● IRON PIN SET
- o RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN S.R. 729
STA. 151 + 00 TO STA. 163 + 00

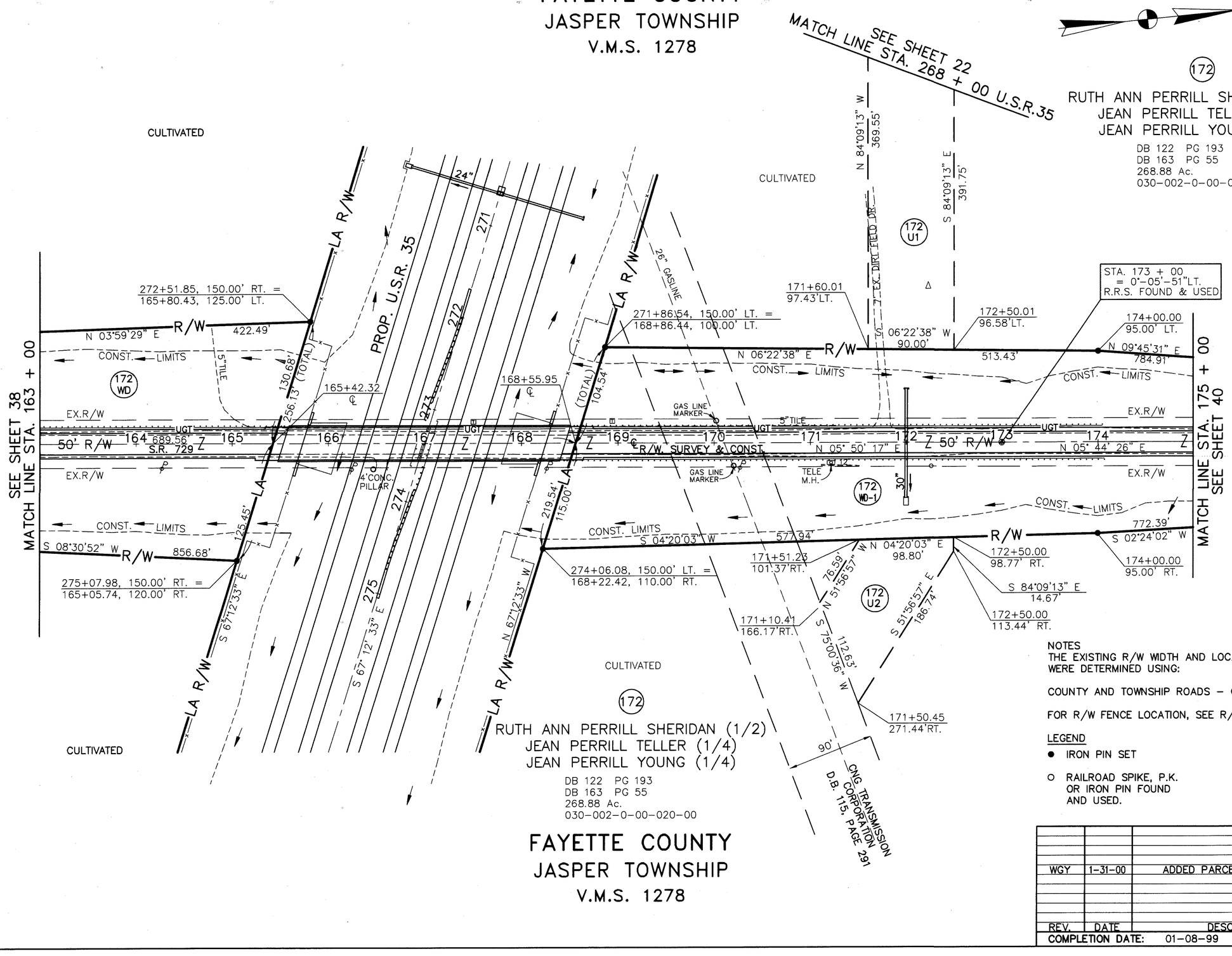
FAY-35-2.57

38 / 40

520
522

Q:\3590\SR729RW1

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278



CULTIVATED

172

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

172

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

NOTES
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS – COUNTY RECORDS

FOR R/W FENCE LOCATION, SEE R/W SHEET 23

LEGEND

- ● IRON PIN SET
- ○ RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

REV.	DATE	DESCRIPTION
WGY	1-31-00	ADDED PARCELS 172U1,172U2

COMPLETION DATE: 01-08-99

SCALE IN FEET: 0, 50, 100

P I D NO. 9078

RIGHT OF WAY PLAN S.R. 729
STA. 163 + 00 TO STA. 175 + 00

FAY-35-2.57

39 / 40

521 / 522

Q:\3590\SR729RW2

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

CULTIVATED

SEE SHEET 39
MATCH LINE STA. 175 + 00

12' PROP. AGGR. FIELD DR.

N 09°45'31" E

R/W 784.91'

CONST. LIMITS

EX.R/W

UGT

176 177 178 179 180 181

50' R/W

℄ R/W, SURVEY & CONST. S.R. 729

N 05° 44' 27" E

EX.R/W

(172 WD-1)

CONST. LIMITS 772.39'

S 02°24'02" W R/W

181+82.98, 40.00' LT. =
108+46.93, 40.00' RT.

END WORK
S.R. 729
STA. 181+44

181+71.08, 50.00' RT. =
109+37.13, 40.00' RT.

CULTIVATED

(172)

RUTH ANN PERRILL SHERIDAN (1/2)
JEAN PERRILL TELLER (1/4)
JEAN PERRILL YOUNG (1/4)

DB 122 PG 193
DB 163 PG 55
268.88 Ac.
030-002-0-00-020-00

FAYETTE COUNTY
JASPER TOWNSHIP
V.M.S. 1278

(161)

V.M.S. 1222
V.M.S. 1278

EX.R/W

105

EX. U.S.R. 35

107 ℄ CONSTR. 106

108

EX U.S.R. 35 CURVE DATA

P.I. STA. 104+54.03
P.C. STA. 100+11.85
P.T. STA. 108+89.29
Δ = 17° 32' 52" LT.
Dc= 2° 00' 00"
R = 2864.93'
T = 442.18'
L = 877.44'

R = 2904.93' ARC L. = 42.95'
CH. L. = 42.95' CH. B. = S 76°30'10" E

STA. 182+17.85 S.R.729 =
STA. 108+81.56 EX. U.S.R. 35

182

109

P.T.

END R/W ACQUISITION
FAY-35-2.57
STA. 181+82.98, S.R. 729

181+77.18
2.56' RT.

S 76°55'35" E
47.84'

110

111

JEFFERSON TWP.
JASPER TWP.

EX.R/W

EX.R/W

V.M.S. 1223
V.M.S. 1278

NOTE
THE EXISTING R/W WIDTH AND LOCATION WERE DETERMINED USING:

COUNTY AND TOWNSHIP ROADS - COUNTY RECORDS
S.H. 29 - SEC. Gpt & Hpt
FAY-35-3.31

LEGEND
- ● IRON PIN SET
- o RAILROAD SPIKE, P.K. OR IRON PIN FOUND AND USED.

REV.	DATE	DESCRIPTION

COMPLETION DATE: 01-08-99

N

SCALE IN FEET 0 50 100

P I D NO. 9078

RIGHT OF WAY PLAN S.R. 729
STA. 175 + 00 TO STA. 183 + 00

FAY-35-2.57

40/40

522/522

Q:\3590\SR729RW3