

Geotechnical Evaluation
GB 1 Subgrade Analysis

I-70 South Trench
W-07-109

Prepared For:

MS Consultants, Inc.
2221 Schrock Road
Columbus, Ohio 43229

January 2010

**GEOTECHNICAL EVALUATION
GB 1 SUBGRADE ANALYSIS**

**I-70 SOUTH TRENCH
W-07-109**

PREPARED FOR:

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JANUARY 2010

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TABLE OF CONTENTS

	Page
TABLE OF CONTENTS	i
1.0 INTRODUCTION	1
2.0 SUBSURFACE INVESTIGATION	3
2.1 Subsurface Investigation Methods	3
2.2 General Geologic Material	4
2.3 General Subsurface Conditions Encountered in the Borings	4
2.4 Groundwater Observations	5
2.5 Laboratory Testing.....	6
3.0 PRELIMINARY PAVEMENT SUBGRADE EVALUATION AND RECOMMENDATIONS.....	7
3.1 GB 1 Analysis Data Input	7
3.2 GB 1 Analysis Results - Entire I-70 South Trench Area	8
3.3 GB 1 Analysis I-70 Eastbound	10
3.4 GB 1 Analysis I-70 Westbound	10
3.5 GB 1 Analysis Ramp F1	10
3.6 GB 1 Analysis Ramp M1	11
3.7 GB 1 Analysis Ramp N1.....	11
3.8 Pavement Subgrade Recommendations.....	11

APPENDICES

Appendix	Description
A	Figures: <ol style="list-style-type: none">1. Site Location Map2. Soil Boring Locations I-70 South Trench3. GB 1 Subgrade Analysis I-70 South Trench (West Portion)4. GB 1 Subgrade Analysis I-70 South Trench (West Central Portion)5. GB 1 Subgrade Analysis I-70 South Trench (East Central Portion)6. GB 1 Subgrade Analysis I-70 South Trench (East Portion)
B	Tables: <ol style="list-style-type: none">1. Soil Boring Locations2. Summary of Topsoil/Existing Pavement Thickness and Observed Water Levels at Boring Locations3. GB 1 Subgrade Analysis Results
C	GB 1 Model Analysis Output - Entire I-70 South Trench Project Area
D	GB 1 Model Analysis Output - Individual Roadway Segments for I-70 South Trench
E	Boring Logs: I-70 South Trench

1.0 INTRODUCTION

The Ohio Department of Transportation (ODOT) proposes to reconstruct the east and south portions of the Interstate (I) I-70/I-71/I-670 Innerbelt located in downtown Columbus, Ohio. This system of highways, bridges, and interchanges extends in a south to north direction along I-71 and its convergence with State Route (SR) 315 from Greenlawn Avenue to West Broad Street and also in a south to north direction from the I-70/I-71 split north along I-71 to the I-670 interchange. The limits of the project extend along I-70 in a west to east direction from a point near Sullivant Avenue to Fairwood Avenue. Reconstruction of the entire system includes the replacement or reconstruction of I-70 East and I-70 West as well as I-71 North and I-71 South within the project area. Replacement or construction of bridges, abutments, and roadways will be included.

The project area for this GB 1 subgrade analysis report is defined as the I-70 South Trench located between the I-70/SR 315 split (West Interchange) and I-70/I-71 East Interchange. The project site location is shown on **Figure 1**, contained in **Appendix A** of this report (all figures are contained in **Appendix A**). The area generally extends in a west to east direction from the railroad overpass east of Whittier Street to Grant Avenue. Subgrade improvements for other segments of the project area will be submitted in separate GB 1 subgrade analysis reports. The proposed mainline I-70 and associated ramps are included. The locations of the proposed South Trench mainline roadways and ramps are shown on **Figure 2**. Soil borings used to analyze subgrade soils and conditions for the area were drilled by DLZ Ohio, Inc. (DLZ). Subsurface investigations were performed in accordance with ODOT's "Specifications for Geotechnical Explorations," dated January 16, 2009, applicable Geotechnical Bulletins distributed by ODOT Office of Geotechnical Engineering, and applicable American Society for Testing and Materials (ASTM) guidelines. Information from borings drilled for retaining walls was also used to analyze subgrade soils for pavement design if the borings were located adjacent to the proposed roadway alignments.

ODOT Geotechnical Bulletin GB 1, titled "Plan Subgrades," dated January 18, 2007, was used to analyze the soils information gathered during the subsurface investigation. Information from the soil boring logs and laboratory analysis results of the soil samples collected from the borings was entered into the GB 1 data analysis spreadsheet provided by ODOT. Results of the GB 1 analysis were used to prepare the recommended subgrade stabilization methods and extent of subgrade repairs that are summarized in the last

section of this document. The following sections provide a general description of the subsurface conditions in the vicinity of the South Trench, GB 1 analysis input parameters, GB 1 analysis results, and subgrade improvement recommendations.

2.0 SUBSURFACE INVESTIGATION

2.1 Subsurface Investigation Methods

Soil borings were drilled by DLZ with either a truck-mounted or all terrain vehicle (ATV)-mounted drill rig. The soil borings that provide relevant subsurface information for the South Trench are located on **Figure 2** through **Figure 6**. **Table 1**, located in **Appendix B**, is a listing of the soil borings used for this section of the project area. Borings are listed in order of the associated segment of mainline roadway or ramp. In addition to the borings drilled within the proposed pavement area, borings drilled for retaining wall design at various locations within the South Trench were used for analysis. If these borings provided pertinent subsurface information and they were near the proposed edge of pavement, they were included in the analysis.

During drilling, field personnel made measurements and observations of subsurface conditions and recorded them on boring logs. Boring logs for the segment of the project discussed in this report are contained in **Appendix E**. After drilling, each boring location was surveyed in the field for location and elevation. The boring locations (northing and easting) were provided by DLZ in Ohio State Plane coordinates. Elevations were provided by DLZ in North American Datum of 1983.

The borings were drilled using rotary-type drilling methods. At regular intervals, disturbed, but representative soil samples, were obtained using a 2-inch outside diameter (OD) split-barrel sampler driven into the soil by blows from a 140-pound hammer free falling 30 inches (Standard Penetration Test [SPT] - ASTM D1586). Driving resistance was recorded in the field and listed on the boring logs in terms of the number of hammer blows for each 6-inch driving interval. The second and third intervals (N_2 and N_3) are entered into the GB 1 analysis spreadsheets and are added together to calculate the number of blows per foot (N). GB 1 analysis also requires the entry of drill rod energy ratio (ER) for each boring. The drill rod energy is expressed as a percent and is associated with N in the following equation:

$$N_{60} = N_m \times (ER/60) \quad \text{where: } N_m = \text{the measured N value } (N_2 + N_3)$$

ER = drill rod energy ratio as a percent for the specific rig used

N_{60} = 60% energy ratio

Three different drill rigs were used to obtain soil samples for the project area. DLZ provided the various ERs for each boring installed. The ERs for the equipment used in this project are listed below:

Notation on GB 1 Analysis	Drill Rod Energy Ratio
A	61
B	62
C	63

2.2 General Geologic Material

The State of Ohio Department of Natural Resources (ODNR), Division of Geological Survey Bulletin 44 indicates that the general area of the site was glaciated by both the Illinoisan- and Wisconsin-age ice sheets, and that the unconsolidated deposits are relatively thick. According to bedrock topography maps of the area, the top of bedrock varies from Elevation 600 to Elevation 650 within the South Trench. This would suggest the depth to bedrock is greater than 75 feet below the existing site grades and would not affect pavement subgrade design.

There are no significant drainage features located within the South Trench area. The topography of the site varies significantly north to south throughout this area as a result of the existing I-70 roadway. The original surface grades in this area were generally between Elevations 750 and 770 prior to construction of the existing I-70 roadway and South Trench. The construction of this depressed roadway created a trench-type condition for portions of the South Trench that are approximately 20 to 30 feet below the surrounding grades.

2.3 General Subsurface Conditions Encountered in the Borings

The following paragraphs provide a general description of the subsurface conditions encountered in the soil borings. Locations of the borings used for the subgrade analysis are shown on the **Figure 2**. For a detailed description of the conditions encountered at each boring location, refer to each individual boring log, contained in **Appendix E**.

Soil borings located within the South Trench area encountered competent glacial till soils or dense granular outwash material. In general, the test borings penetrated approximately 10 to 136 feet below existing grades. Surface materials in the area included topsoil, concrete pavement, or gravel. **Table 2** is a summary of the topsoil, pavement, and pavement base material thicknesses encountered at each boring location in the project area as reported on the boring logs (boring logs are included in **Appendix E**). Surface material thicknesses were noted in order to establish the sampling interval depths of subgrade soils to be input into the GB 1 spreadsheet analysis since topsoil, pavement, and aggregate base materials are not applicable to the analysis.

Soils information used for the pavement subgrade analysis was limited to the first 6 feet of soil below proposed subgrade elevations by the GB 1 analysis spreadsheet. Primarily fine grained (generally cohesive) soils were encountered near surface. The SPT blow counts were generally high and correspond to relative densities of dense or better for granular soils, and for fine grained soils the consistency would typically be correlated as stiff to very stiff. Bedrock was not encountered within the first 6 feet of finished subgrade in any of the borings. The moisture content of the soil samples exhibiting cohesive properties was typically -6 to +6 points from optimum; with optimum moisture content based on the soil classification rather than site-specific laboratory analysis.

2.4 Groundwater Observations

The elevation of groundwater observed in boreholes during drilling was noted on each boring log. Water level information is included in **Table 2**. Observed water elevations in the borings ranged from 9.2 to 43.3 feet below ground surface (bgs). Several borings were reported as "dry" to depths up to 20 feet.

The true groundwater levels cannot be discerned from the information provided. Short-term water level readings in boreholes, especially in cohesive soils, are an indication, but not an accurate measurement of the groundwater elevation in an area. The long-term groundwater elevations would need to be confirmed by installing monitoring wells or piezometers and performing long-term (seasonal) groundwater measurements.

2.5 Laboratory Testing

Laboratory testing on recovered soil samples consisted of ODOT soil classification, moisture content, Atterberg limits, and grain size analysis. Tests results are reported on the boring logs contained in **Appendix E** and within the GB 1 analysis data discussed in Section 3.0.

3.0 PRELIMINARY PAVEMENT SUBGRADE EVALUATION AND RECOMMENDATIONS

3.1 GB 1 Analysis Data Input

The project area for this report is defined as the immediate area of the proposed South Trench for I-70. The proposed mainline I-70 and ramps were included. **Table 1** included in **Appendix B** shows the soil borings used for the subgrade analysis for each segment evaluated. The complete project area was analyzed on one GB 1 analysis spreadsheet in addition to each segment of the project being analyzed on separate GB 1 analysis spreadsheets.

A GB 1 analysis required data input into ODOT provided spreadsheets that will generate recommended subgrade stabilization measures, where necessary. Input data required from each soil boring are as follows:

- Boring number
- Boring location
- Sample depths (from proposed subgrade elevation)
- N_2 and N_3 (blow counts for second and third 6-inch intervals)
- drill rig ER
- liquid limit and plastic limit
- percent silt and percent clay
- moisture content
- soil classification.

The GB 1 analysis spreadsheet calculates the following information for each boring location:

- N_m - number of blows per foot required to drive the sample tube through the soil.
 $N_M = N_2 + N_3$, sum of blow counts for the second and third 6-inch increments for each split spoon sample drive
- N_L - the lowest N_{60} value recorded in the top 6 feet of material

-
- $N_{60} - N_M$ corrected to an equivalent drill rod energy of 60 percent
 - PI - Plasticity Index
 - P_{200} - percent of material passing No. 200 sieve
 - M_{opt} - optimum moisture content (based on soil type)
 - Problems with subgrade soil relative to the soil classification
 - Problems with subgrade soil relative to the soil moisture or soil strength estimated from blow counts
 - Possible subgrade treatments (undercutting, cement stabilization [CS], or lime stabilization [LS]).

Sample depths presented on the boring logs were changed to depth below design subgrade prior to input into the GB 1 analysis spreadsheet. This will establish the depth of undercutting or depth of stabilization needed from design subgrade rather than from a reference elevation of the top of each boring (existing grade at the time of drilling). Soil sample results for soils that will be cut out prior to establishing the design subgrade elevations were not entered into the GB 1 analysis.

3.2 GB 1 Analysis Results - Entire I-70 South Trench Area

As required by GB-1, the following is a tabulation of the average results for the entire I-70 South Trench project area:

Average NL for entire South Trench area	22.0
Average PI to the nearest whole number	10
Average design California Bearing Ratio (CBR) to the nearest whole number (as an average not a percent)	9
Average moisture content to the nearest whole number	10

The GB 1 analysis spreadsheet output for the entire South Trench project area is contained in **Appendix C**. The project area was also broken up into segments to complete individual analyses for each roadway segment and ramp. The GB 1 analysis output for each individual segment is contained in **Appendix D**. **Table 3** presents the GB 1 analysis results of the individual segments. The calculated design CBRs ranged from 8 to 11 as indicated in **Table 3**.

The designer, based on the results of subsurface exploration, is responsible for identifying the method, location, and dimensions (including depth) of subgrade stabilization in the plans. GB 1 is to be used as general guidance. Limits for subgrade stabilization estimated in this GB 1 analysis should be verified and adjusted in the field based on proof rolling and visual observation. Undercutting estimates and treatment options were based on the following parameters:

- Soils at a depth of 6 feet or greater below finished subgrade were not considered in the subgrade improvements to be implemented since the soils were too deep.
- Maximum undercut depth was set at 6 feet from design subgrade since the undercut will be filled with competent material.
- Where the GB 1 analysis indicated an undercut depth, the undercut depth was applied only to the sample interval if the underlying soils were indicated to not need undercutting by the GB 1 analysis.
- Where the GB 1 analysis indicated the top sample in the boring required undercutting, the undercut was started at the top of the sample interval needing cut.

The soils classifications data for the project area identified in the boring program were identified as type A-1a, A-1b, A-2-4, A-3, A-3a, A-4a, A-4b, A-6a, A-6b, and A-7-6. Out of the identified materials, only soil type A-4b will be removed or stabilized based on soil type alone.

The moisture content of cohesive soils has a significant effect on the physical properties of the soil. Moisture content reported on the boring logs only represents the moisture content during drilling and sampling. Actual moisture content during construction may vary

greatly from the measured soil moisture content reported. Undercut areas required by GB 1 analysis presented in this report are based on soil type and excessive moisture in the samples collected. The extent and need for subgrade improvement is dependent on the actual subgrade conditions during site preparation and construction. Each area of subgrade for proposed pavements must be confirmed in the field by proof rolling to identify the need for subgrade stabilization.

The following sections summarize the results of the GB 1 analyses performed for individual roadway segments or ramps. As described below, no subgrade stabilization methods are evidenced based on the results of the GB 1 output for the South Trench.

3.3 GB 1 Analysis I-70 Eastbound

Eleven soil borings were used to determine subgrade stabilization needs for I-70 eastbound reconstruction. The calculated design CBR for this portion of the project area is 9.

No areas for undercutting or subgrade stabilization were identified. Boring B-031 on **Figure 5** shows the presence of A-4b soils located at depths greater than 3 feet below proposed subgrade and therefore these soils need not be removed.

3.4 GB 1 Analysis I-70 Westbound

Eleven soil borings were used to determine subgrade stabilization needs for the proposed I-70 westbound reconstruction in the project area. The calculated design CBR for this portion of the project area is 9. No areas for undercutting or subgrade stabilization were identified.

3.5 GB 1 Analysis Ramp F1

Ramp F1 is located south of the I-70 mainline on the west end of the South Trench. The majority of Ramp F1 is bridge section. One section of Ramp F1, approximately from Station 264+50 to Station 267+50, is not a bridge section. Boring B-018 was used to determine subgrade stabilization needs for the proposed Ramp F1. Ramp F1 is shown on **Figures 3 and 4**. The final design alignment and grade for Ramp F1 shows approximately 9.7 feet of fill material will be added to the ramp subgrade at boring B-018. Since the

amount of fill is greater than 8 feet, the GB 1 analysis spread sheet did not calculate an average group index or a design CBR for the ramp segment. According to the GB 1 analysis, undercutting or other stabilization methods are not required for the 300-foot section of Ramp F1 in the project area.

3.6 GB 1 Analysis Ramp M1

One soil boring (B-022) was used to determine subgrade stabilization needs for the proposed Ramp M1 in the project area. Ramp M1 is shown on **Figures 3 and 4**. The calculated design CBR for this portion of the project area is 11. According to the GB 1 analysis, undercutting or other subgrade stabilization methods are not required for Ramp M1. Only the eastern portion of Ramp M1 is pavement construction from approximately Station 625+50 to 631+00. The western portion of Ramp M1 is a bridge section.

3.7 GB 1 Analysis Ramp N1

One soil boring (B-043) was used to determine subgrade stabilization needs for the proposed Ramp N1 in the project area. Ramp N1 is shown on **Figure 6**. The calculated design CBR for this portion of the project area is 8. According to the GB 1 analysis, undercutting or other subgrade stabilization methods are not required for the portion of Ramp N1 located west of Grant Avenue. The GB 1 analysis for the portion of Ramp N1 located east of Grant Avenue is contained in a separate GB 1 analysis document for the Southeast Interchange.

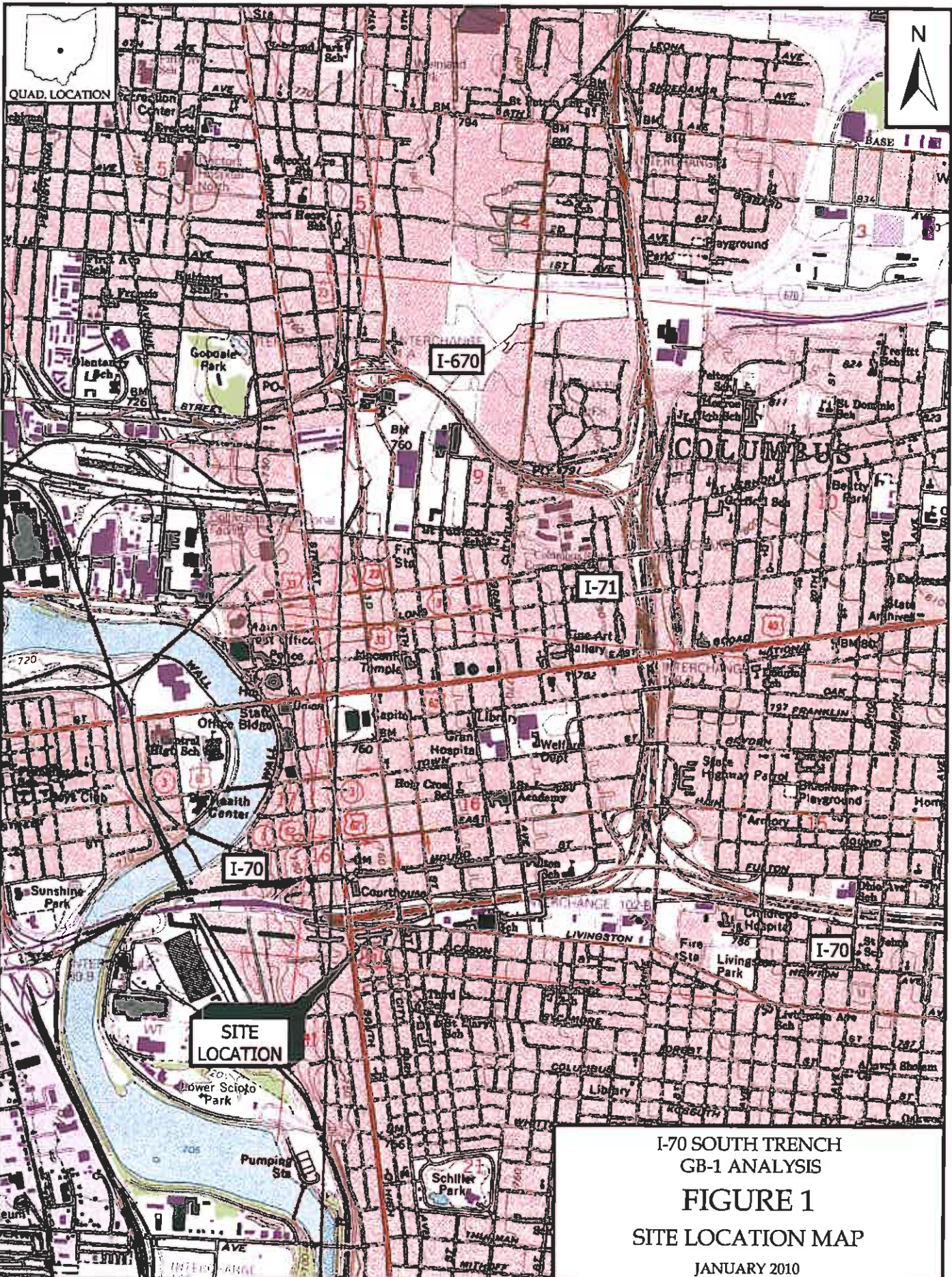
3.8 Pavement Subgrade Recommendations

All recommendations for subgrade stabilization improvement/treatments for the South Trench area are based on the Subgrade Version 9.09 as provided by ODOT and described in GB 1. Undercutting is not required in the South Trench project area (22 boring locations). Cement stabilization is an option in one area of westbound I-70 (boring B-034) but is not considered practical due to the limited area needing cement stabilization. Lime stabilization was not listed as an option in any areas of the South Trench project site.

APPENDIX A

FIGURES

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SOURCE:
7.5 MINUTE COLUMBUS U.S.G.S. QUADRANGLE MAP

I-70 SOUTH TRENCH
 GB-1 ANALYSIS
FIGURE 1
 SITE LOCATION MAP
 JANUARY 2010

BURGESS & NIPLE
 Engineers • Environmental Scientists • Geologists

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3000 Road Road
Columbus, Ohio 43220

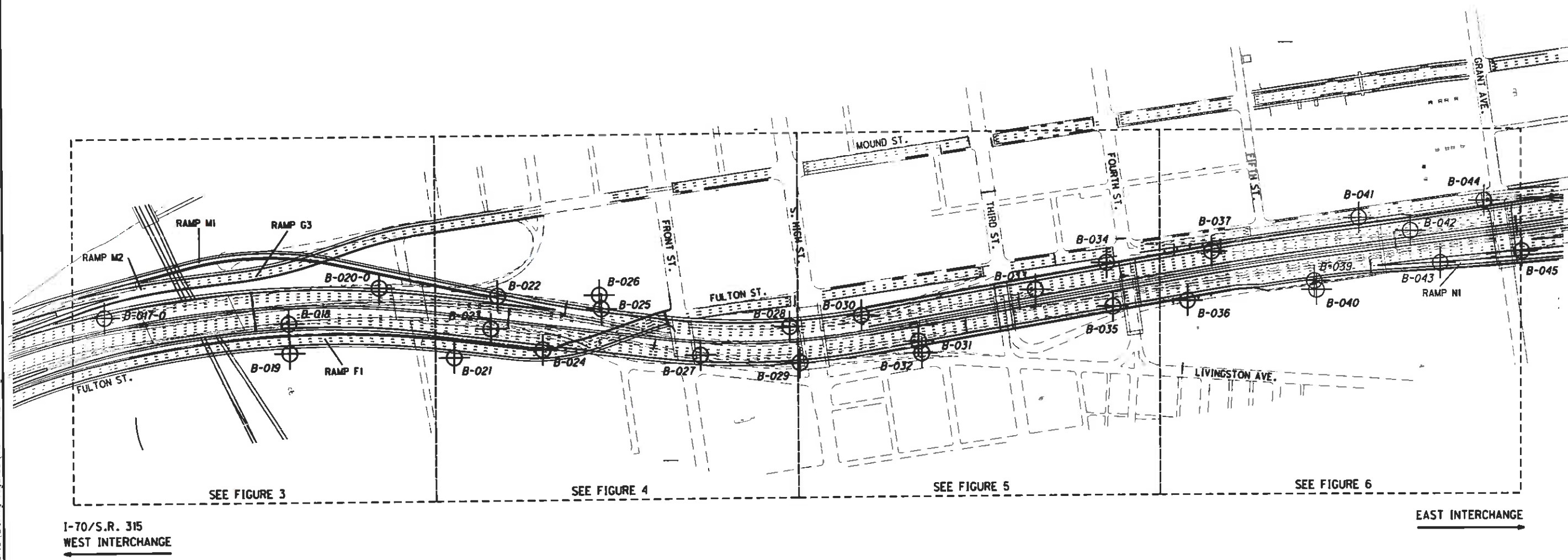
DESIGNED	VEA	CHECKED	VEA
DRAWN	RVZ	REVIEWED	DATE
STRUCTURE FILE NUMBER			

FRANKLIN COUNTY

SOIL BORING LOCATIONS
CB-1 SUBGRADE ANALYSIS
I-70 SOUTH TRENCH

W-07-109
FIGURE 2

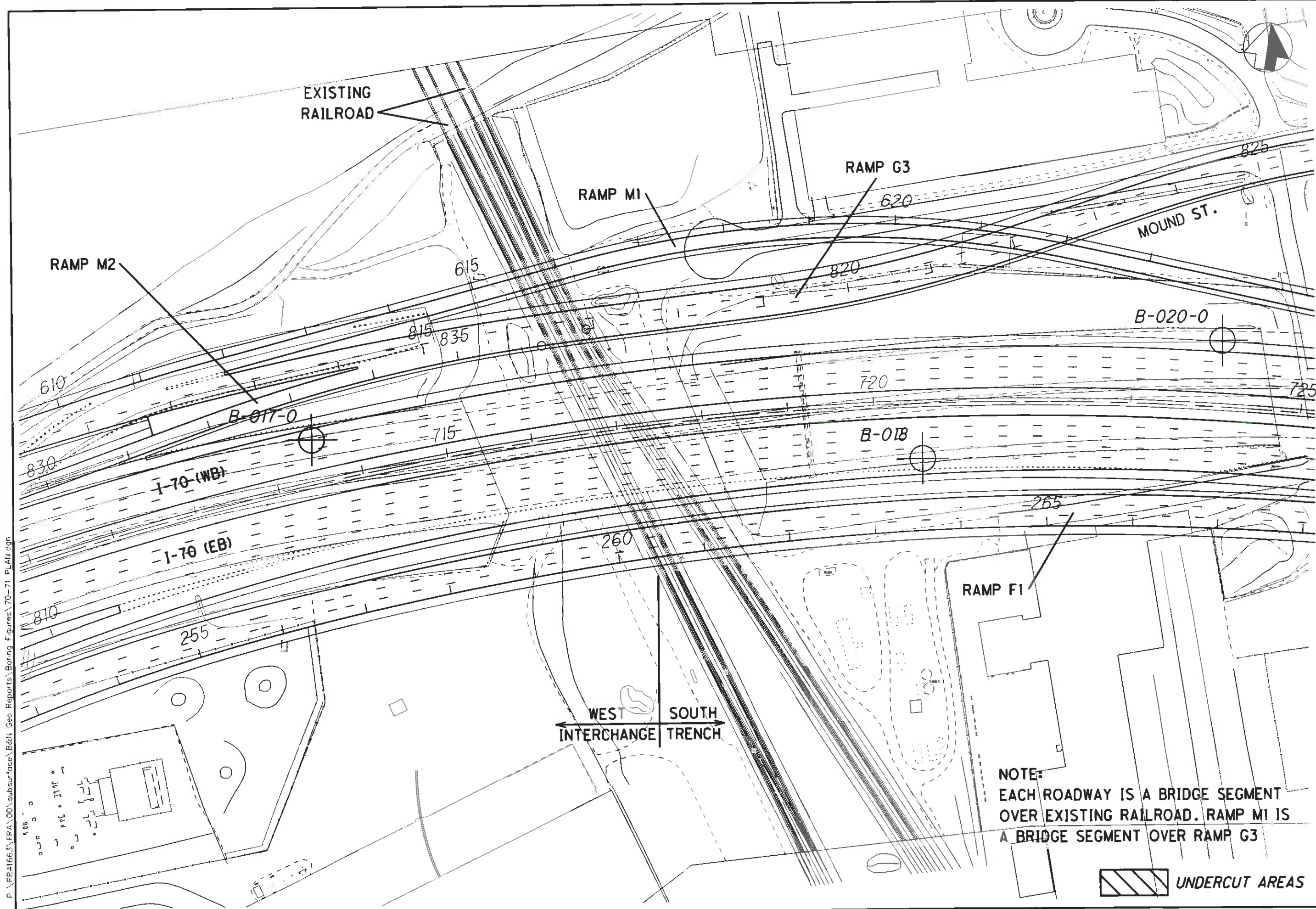
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SOIL BORING LOCATIONS

PLAN

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EXISTING RAILROAD

RAMP M2

RAMP M1

RAMP G3

MOUND ST.

B-017-0

B-018

B-020-0

I-70 (WB)

I-70 (EB)

RAMP F1

WEST INTERCHANGE TRENCH | SOUTH INTERCHANGE TRENCH

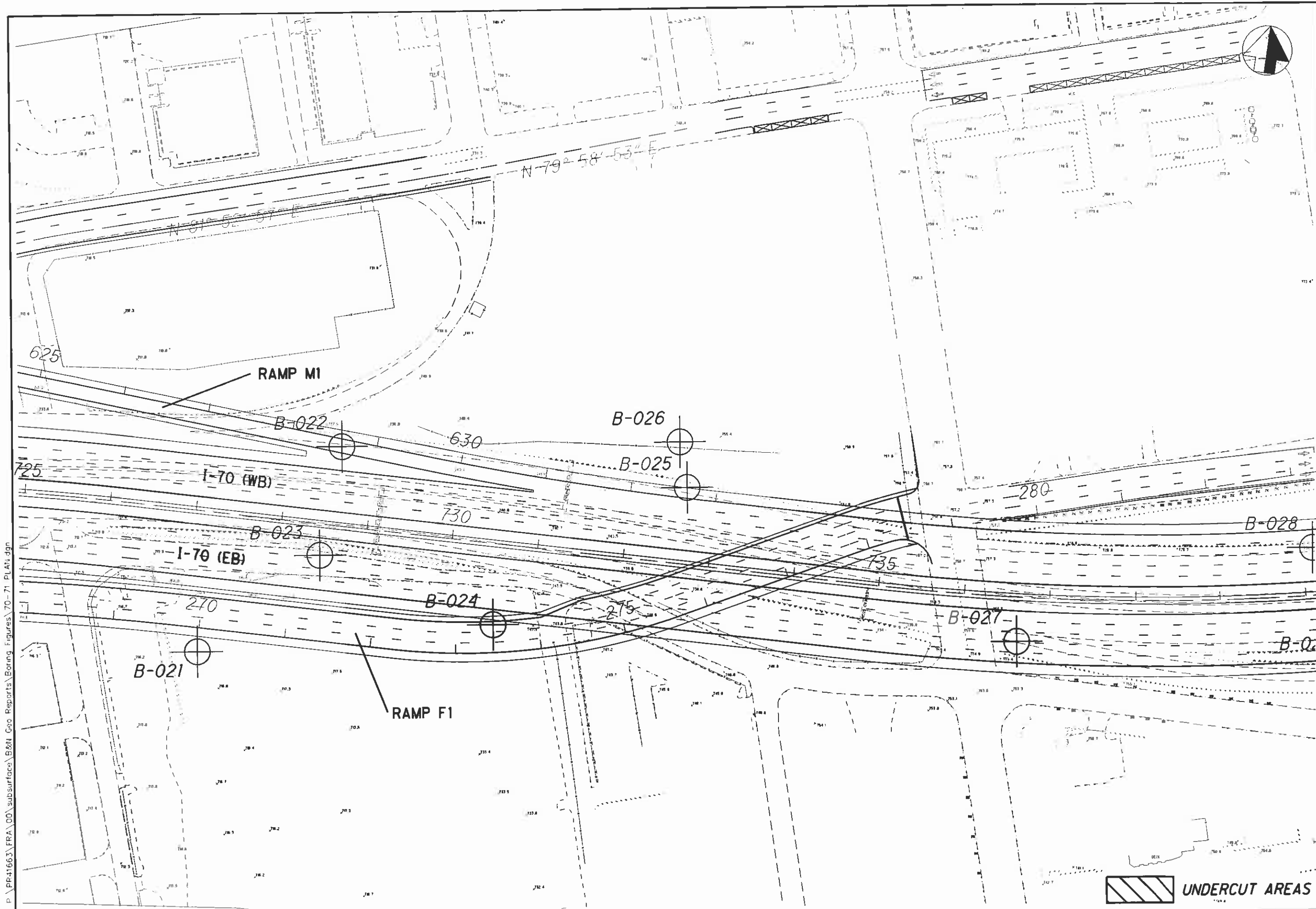
NOTE:
EACH ROADWAY IS A BRIDGE SEGMENT OVER EXISTING RAILROAD. RAMP M1 IS A BRIDGE SEGMENT OVER RAMP G3

 UNDERCUT AREAS

FRANKLIN COUNTY	DESIGNED	DATE	REVIEWED	DATE	STRUCTURE FILE NUMBER	DRAWN	DATE	REVISED	DATE	REVISED	DATE
	VEA			RVZ					VEA		
SOIL BORING LOCATIONS CB-1 SUBGRADE ANALYSIS WEST PORTION											
W-07-109 FIGURE 3											
2 / 5											

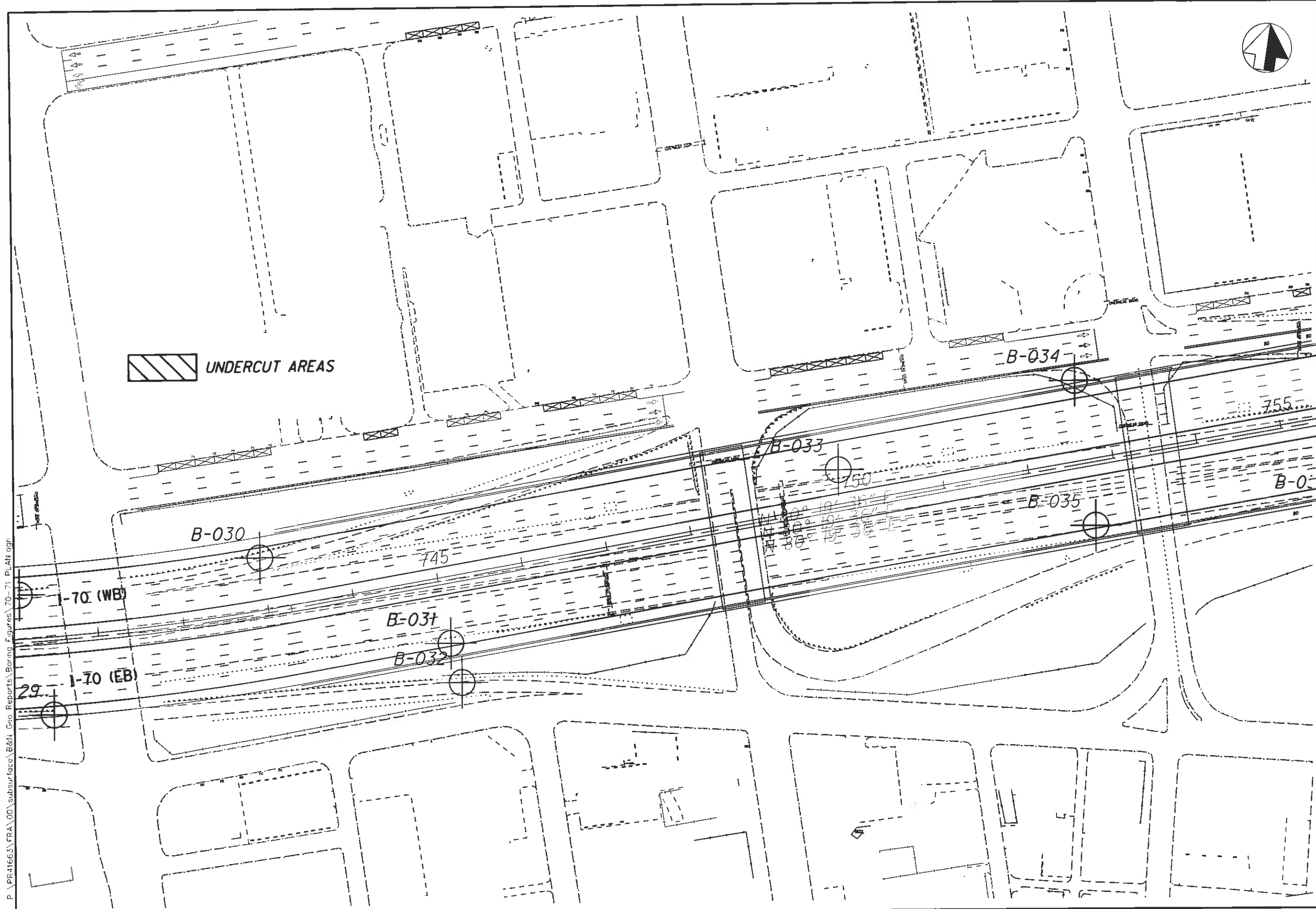
BRUNNEN & MUELLER
5000 Road Road
Columbus, Ohio 43220

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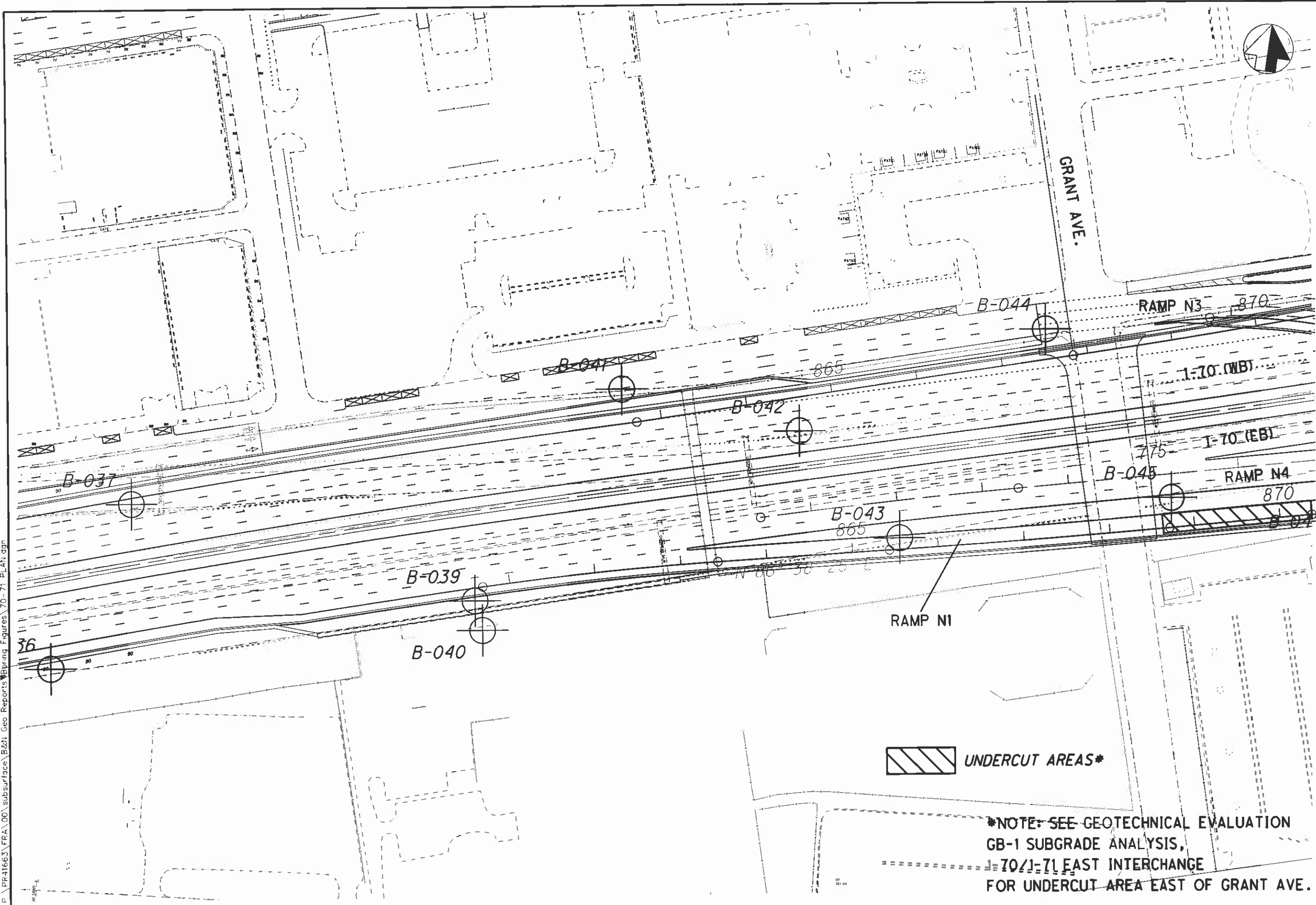
	
DATE	REVIEWED
STRUCTURE FILE NUMBER	REVISED
VEA	RVZ
VEA	VEA
FRANKLIN COUNTY	
SOIL BORING LOCATIONS CB-1 SUBGRADE ANALYSIS WEST CENTRAL PORTION	
W-07-100 FIGURE 4	
3 / 5	
	

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 5000 Road Road Columbus, Ohio 43220 BRUNNEN & NIPPLE	REVIEWED	DATE
	DRAWN	DATE
VEA	VEA	3-D TYPE FILE NUMBER
VEA	VEA	
FRANKLIN COUNTY		
SOIL BORING LOCATIONS		
CB-1 SUBGRADE ANALYSIS		
EAST CENTRAL PORTION		
W-07-109	FIGURE 5	
4	5	

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 UNDERCUT AREAS*

*NOTE: SEE GEOTECHNICAL EVALUATION
GB-1 SUBGRADE ANALYSIS,
I-70/I-71 EAST INTERCHANGE
FOR UNDERCUT AREA EAST OF GRANT AVE.

FRANKLIN COUNTY	DESIGNED	DATE	BURDICK & WELLS 5005 Reed Road Columbus, Ohio 43220
	VEA	STRUCTURE FILE NUMBER	
SOIL BORING LOCATIONS GB-1 SUBGRADE ANALYSIS EAST PORTION	DRAWN	RYZ	REVISED
	VEA	VEA	VEA
W-07-100 FIGURE 6	5 / 5		

APPENDIX B

TABLES

Table 1
Soil Boring Locations

Project Segment	Station Range	Total Number of Borings	Boring I.D.s
Entire South Trench area	N/A	22	
I-70 (Eastbound)	716+00 - 768+00	11	B-018, B-023, B-024, B-027, B-029, B-031, B-035, B-036, B-039, B-043, B-045
I-70 (Westbound)	716+00 - 768+00	11	B-020, B-022, B-025, B-028, B-030, B-033, B-034, B-037, B-041, B-042, B-044
Ramp F1	263+50 - 267+50	1	B-018
Ramp M1	612+00 - 631+00	1	B-022
Ramp N1	763+00 - 768+20	1	B-043

Soils information from three borings (B-018, B-022, and B-043) was used for subgrade analysis on more than one road segment or ramp.

Table 2
Summary of Topsoil/Existing Pavement Thickness and Observed Water Levels at Boring Locations

Boring I.D.	Topsoil Thickness (inches)	Concrete Thickness (inches)	Asphalt Thickness (inches)	Base Material Thickness (inches)	Depth to Water (feet)	Total Depth (feet)
I-70 Mainline (Eastbound)						
B-018	--	10	--	4	15.0	17.5
B-023	--	12	--	6	19.1	35
B-024	--	--	--	--	16.7	111.5
B-027	--	18	--	11	>14	14
B-029	--	16	--	11	21.0	136.5
B-031	8	--	--	--	9.5	60
B-035	--	18	--	3	9.2	15
B-036	--	20	--	4	14	115.6
B-039	--	16	--	5	>17.5	17.5
B-043	--	18	--	5	14.4	115
B-045	--	8	--	5	>15	15
I-70 Mainline (Westbound)						
B-020	--	16	--	4	>20	20
B-022	--	16	--	4	>20	20
B-025	--	7	--	7	39	59.3
B-028	--	12	--	6	>10	10
B-030	--	11	--	6	13.6	111
B-033	--	--	--	2	9.2	15
B-034	3	--	--	--	43.3	135.5
B-037	--	17	--	5	25.6	130
B-041	--	8	--	5	37.1	135
B-042	2	--	--	--	>15	15
B-044	--	6	--	10	20.8	131.5
Ramp F1						
B-018	--	10	--	4	15.0	17.5
Ramp M1						
B-022	--	16	--	4	>20	20
Ramp N1						
B-043	--	18	--	5	37	115

Soils information from three borings (B-018, B-022, and B-043) was used for subgrade analysis on more than one road segment or ramp.

Table 3
GB 1 Subgrade Analysis Results

Project Segment	Number of Soil Borings	Average N ₆₀	Average N _L	Average PI	Average Moisture	Average Opt. Moist.	Average Group Index (GI)	Design CBR
Entire South Trench Area	22	40.9	22.0	9.9	10.0	10.1	3	9
I-70 Mainline Eastbound	11	40.6	22.6	9.7	10.2	10.2	3	9
I-70 Mainline Westbound	11	41.2	21.3	10.1	10.0	10.0	3	9
Ramp F1**	1							
Ramp M1	1	28.2	13.0	4.7	12.2	10.8	1.5	11
Ramp N1	1	72.3	30.0	8.5	10.0	11.5	4	8

** - The GB1 analysis spreadsheets did not calculate an average Group Index or a Design CBR for Ramp F1 since all the soil samples for that ramp are under approximately 10 feet of fill.

APPENDIX C

GB 1 MODEL ANALYSIS OUTPUT - ENTIRE I-70 SOUTH TRENCH PROJECT AREA

APPENDIX D

**GB 1 MODEL ANALYSIS OUTPUT - INDIVIDUAL ROADWAY SEGMENTS
FOR THE I-70 SOUTH TRENCH**

APPENDIX E

BORING LOGS
I-70 SOUTH TRENCH

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-017-0

Location: Sta. 713+49.89, 51.13 ft Lt. of I-70 CL

Date Drilled: 7/25/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Press / Core	Hand Penetrometer (tsf)	FIELD NOTES	WATER OBSERVATIONS	Graphic Log	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP					
											% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
18	735.9	5	9	1			2.5	Asphalt Concrete - 6" Portland Cement Concrete - 11" Aggregate Base - 4" Very stiff brown SILT AND CLAY (A-6a), little fine to coarse sand, little gravel; moist.	None None	Asphalt Concrete - 6" Portland Cement Concrete - 11" Aggregate Base - 4"	40	47	6	7								
3.5	734.2	2	8	2				Loose to medium dense brown GRAVEL WITH SAND (A-1-b), trace silt; damp.		Loose to medium dense brown GRAVEL WITH SAND (A-1-b), trace silt; damp.	3	9	10	33	45							
6.5	731.2	3	5	3				Medium stiff to stiff brown SILTY CLAY (A-6b), trace to little fine to coarse sand, trace gravel; damp to moist.		Medium stiff to stiff brown SILTY CLAY (A-6b), trace to little fine to coarse sand, trace gravel; damp to moist.	4	5	15									
9.5	728.2	4	4	4				Stiff to very stiff gray CLAY (A-7-6), trace fine to coarse sand, trace gravel; moist.		Stiff to very stiff gray CLAY (A-7-6), trace fine to coarse sand, trace gravel; moist.	5	5	4	14								
13.5	724.2	5	5	5				Silt brown and gray SILTY CLAY (A-6b), little fine to coarse sand, little gravel; moist.		Silt brown and gray SILTY CLAY (A-6b), little fine to coarse sand, little gravel; moist.	6	2	4	14								
15.0	722.7	9	16	7				Bottom of Boring - 15.0'		Bottom of Boring - 15.0'	7	5	6	16								

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-018 Location: Sta. 720+55.54, 63.68 ft Rt. of I-70 CL Date Drilled: 7/8/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive Press / Core	Hand Penetro-meter (Isf)	FIELD NOTES	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP								
									% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay							
1.2	740.4						Asphalt Concrete Pavement - 10" Aggregate Base - 4"															
1.5	739.2						FILL: Very stiff brown SILTY CLAY (A-6b), little to some fine to coarse sand, little gravel; contains rock fragments; moist.															
2.0		9	17	1		2.5																
2.5		12	9																			
3.0		8	6	2		2.75																
3.5		13	11																			
4.0		10	8	3		4.0																
4.5	733.4	8	10																			
5.0	731.9	5	8	4		-																
5.5		5	5																			
6.0		7	4	5																		
6.5		6	7			3.0																
7.0	729.4	4	6	6																		
7.5		7	8																			
8.0		6	5			1.5																
8.5		2	3	6																		
9.0		3	12																			
9.5		4	7																			
10.0		2	4	7		2.75																
10.5	722.9	7	12																			
11.0		4	7																			
11.5		7	12																			
12.0		4	7																			
12.5		7	12																			
13.0		4	7																			
13.5		7	12																			
14.0		4	7																			
14.5		7	12																			
15.0		4	7																			
15.5		7	12																			
16.0		4	7																			
16.5		7	12																			
17.0		4	7																			
17.5		7	12																			
18.0		4	7																			
18.5		7	12																			
19.0		4	7																			
19.5		7	12																			
20.0		4	7																			
20.5		7	12																			
21.0		4	7																			
21.5		7	12																			
22.0		4	7																			
22.5		7	12																			
23.0		4	7																			
23.5		7	12																			
24.0		4	7																			
24.5		7	12																			
25.0		4	7																			

@ 16.0'-17.5'; contains wood fragments.

Bottom of Boring - 17.5'

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-022

Location: Sta. 728+59.94, 91.57 ft Lt. of I-70 CL

Date Drilled: 7/24/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	FIELD NOTES	WATER OBSERVATIONS	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ●		
									% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay	
738.0							Water seepage at None Water level at completion: None									
<p>FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.</p> <p>DESCRIPTION</p>																
1.7	736.3	9		1	1.5	Asphalt Concrete - 6" Portland Cement Concrete - 10" Aggregate Base - 4"										
3.0	735.0	7	12	2	-	FILL: Stiff brown SILT AND CLAY (A-6a), some fine to coarse sand, little gravel; damp.										
4.5	733.5	6	7	3	2.0	FILL: Stiff brown GRAVEL WITH SAND AND SILT (A-2-4), some silt, trace clay; moist.										
6.0	732.0	18	15	4		FILL: Stiff brown SANDY SILT (A-4a), little gravel, trace clay; contains brick and asphalt fragments; damp.										
7.5	730.5	21	16	5		FILL: Dense gray GRAVEL (A-1-a); damp.										
11.0	727.0	5	11	6	3.0	FILL: Very stiff dark brown SILTY CLAY (A-8b), some fine to coarse sand, trace gravel; moist.										
		7		7	3.0	@ 9.0'-10.5', contains small brick fragments.										
		27	11	8		FILL: Dense dark gray SANDY SILT (A-4a), some gravel; damp.										
		3	1	9		@ 13.5'-15.0', loose, brown.										
16.0	722.0	8		10		Medium dense brown COARSE AND FINE SAND (A-3a), little silt, trace gravel; damp.										
		5	13	11												
18.5	719.5	4		12	2.25	Very stiff gray SILTY CLAY (A-6b), trace fine to coarse sand; moist.										
20.0	718.0	5	14	13												
		6		14		Bottom of Boring - 20.0'										

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-023

Location: Sta. 728+48.63, 36.10 ft. Rt. of I-70 CL

Date Drilled: 8/20/2008 to 9/2/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Hand Penetrometer (tsf)	FIELD NOTES	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ●										
									% Aggregate	% C Sand	% M. Sand	% F. Sand	% Silt		% Clay	PL	LL	Blows per foot - ○ / Non-Plastic - NP						
697.0							Water seepage at: 25.0' Water level at completion: 19.1'																	
		18, 32, 33			11		Advanced boring using 3.25" diameter hollowstem augers.																	
		7, 31, 36	18		12		Very dense brown COARSE AND FINE SAND (A-3a), some silt, some gravel, wet. @ 26.0'-27.5', contains turpentine odor.																	
		18, 23, 29	17		13			@ 31.0'-35.0', gray, little silt, little gravel.																
35.0	687.0	18, 28, 35	15		14		Bottom of Boring - 35.0'																	

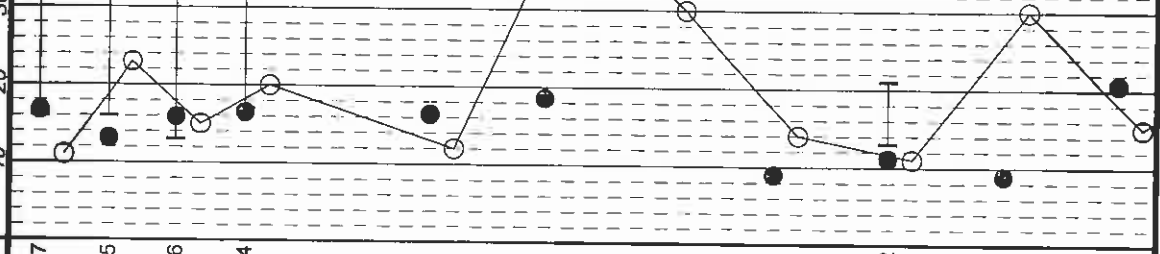
Client: ms consultants Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-024 Location: Sta. 730+58.01, 93.53 ft Rt. of I-70 CL

Date Drilled: 7/1/2008 to 7/2/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 28.5'-80.0' Water level at completion: 16.5' (prior to coring) 10.7' (includes drilling water) FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP						
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay					
743.4		3																	
		4	11	1	--														
		7	12	2	4.5+														
		12	11	3	4.0														
		5	7	4	3.75														
5		4	8	5															
		6	14	6															
		14	14	7															
		21	22	8	4.5														
		22	9	9															
		17	14	10															
		14	15	11															
15.5	727.9		1	12															
		4	7	13															
		7	7	14	3.5														
		4	7	15															
		2	4	16	3.0														
		4	7	17															
20			18	18															
		7	13	19															
		13	16	20															
		3	7	21															
		7	8	22	3.25														
25	718.4		18	23															



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-024

Location: Sta. 730+58.01, 93.53 ft Rt. of I-70 CL

Date Drilled: 7/1/2008 to 7/2/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 28.5'-80.0' Water level at completion: 16.5' (prior to coring) 10.7' (includes drilling water) FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP PL — LL 10 20 30 40	
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
80	668.4	39 50/3	9	23		Very dense brown and gray GRAVEL (A-1-a), some fine to coarse sand, trace silt; wet. @ 78.5', 3.0 feet sand heave. @ 80.0'-90.0', difficult drilling; possible cobbles.		55	36	5	4			50+
85		50/0	0	24										50+
90		50/0	0	25		@ 88.5', 1.0 foot sand heave. @ 90.0' - 91.5', core loss. @ 91.5' - 91.8', encountered igneous-plutonic cobble/boulder; likely peridotite or gabbro.								50+
91.8	651.6	Core 18"	Rec 0"	RQD 0%	R1	Interbedded Shale (90%) and Limestone (10%) RQD 60%, LOSS 26%; Shale, blue-gray, highly weathered, weak, laminated, slightly calcareous, contains abundant pyritic inclusions, moderately to highly fractured; Limestone, light gray, moderately weathered, moderately strong to strong, fractured. @ 97.7' - 98.1', qu = 1650 psi								
100	643.4	Core 120"	Rec 116"	RQD 81%	R2									

Client: ms consultants

Project: FRA-70-8.93

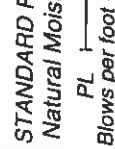
Job No. 0221-1004.01

LOG OF: Boring B-026

Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL

Date Drilled: 8/1/2008 to 8/6/2008

Depth (ft)	Elev. (ft)	Blows per ft	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP			
							% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay				
0.3	754.0					Topsoil - 4"										
2.0	753.7					Stiff brown SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; moist.										
3.5	750.5					Soft brown SANDY SILT (A-4a), some fine to coarse sand, little gravel; wet.										
5						Very stiff to hard brown CLAY (A-7-6), little to some fine to coarse sand, trace to little gravel; moist.										
10						@ 8.5'-10.0', soft, wet.										
15						@ 13.5', becomes gray. @ 13.5'-15.0', stiff.										
18.5	735.5					Very stiff to hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; damp to moist.										
20																
25	729.0															



Client: ms consultants Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-026 Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL Date Drilled: 8/1/2008 to 8/6/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetrometer (tsf)	DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP	
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
55	704.0	21 23 50/5	17	19		4.5+	Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace to little gravel; contains sand seams; damp.		10	13	---	22	35	20	50+
57.0	697.0								30	41	---	21	---	8	NP
60		19 30 44	18	20		4.5	Very dense gray GRAVEL WITH SAND (A-1-b), trace to little silt; wet.								78
65		29 50/4	2	21											50+
67.0	687.0														
70		8 20 23	2	22		4.5	Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace to little gravel; damp to moist.								
75	679.0	13 50/5	10	23		4.5+			9	10	---	19	40	22	50+

Client: ms consultants

Job No. 0221-1004.01

Project: FRA-70-8.93

Date Drilled: 7/24/2008

Location: Sta. 740+11.22, 55.30 ft Lt. of I-70 CL

LOG OF: Boring B-028

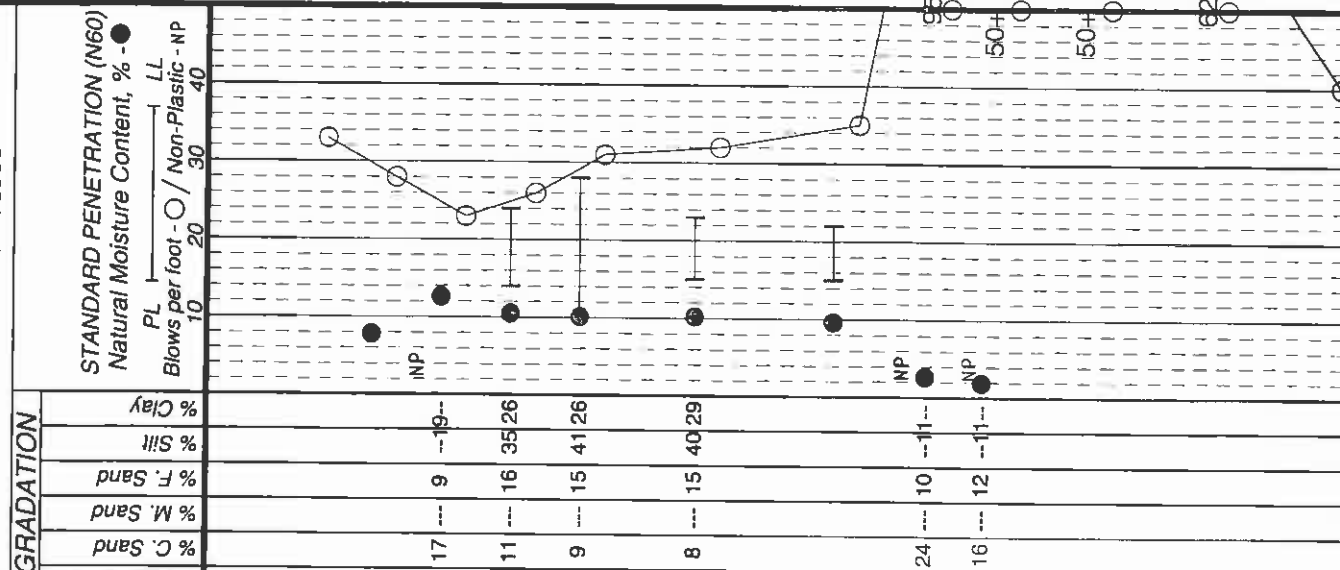
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	Graphic Log	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP						
									% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
1.5	730.2						Asphalt Concrete - 12" Aggregate Base - 6"														
6.0	725.7	10					FILL: Loose to medium dense brown GRAVEL (A-1-a), little to some fine to coarse sand, little silty clay; moist.		65	16	—	6	—	13		●					
		20		1							70	14	—	5	—	11		○			
		22		2							34	35	—	14	—	17		●			
		47		3							84	5	—	3	—	8		●			
8.5	723.2						FILL: Dense brown GRAVEL WITH SAND (A-1-b), little silt; contains brick fragments and silty clay seams; damp.														
10.0	721.7						FILL: Very dense gray GRAVEL (A-1-a), trace fine to coarse sand; contains few silty clay seams; damp.														
							Bottom of Boring - 10.0'														

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Plasticity Index - ○ / Non-Plastic - NP						
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay					
2.3	740.0	15	7	1		Asphalt Concrete Pavement - 7" Portland Cement Concrete - 9" Aggregate Base - 11"	[Hatched]												
5.0	736.3	10	6	2	4.5	POSSIBLE FILL: Medium dense to dense brown GRAVEL WITH SAND (A-1-b), little to some silt; damp.	[Gravel]	55	17	9	19								
7.5	734.8	15	14	3	4.5	Hard gray SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.	[Silt]	12	11	16	35	26							
10.0	732.3	15	12	4	4.5	Hard gray SILTY CLAY (A-6b), some fine to coarse sand, trace gravel; damp to moist.	[Clay]	9	9	15	41	26							
15.0	727.3	15	18	5	4.5	Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp. @ 11.5'-21.0', difficult drilling.	[Silt]	8	8	15	40	29							
21.0	721.3	21	6	6		Very dense gray GRAVEL (A-1-a), some fine to coarse sand, trace to little silt; damp. @ 18.5'-18.9', rock fragments; possible cobble blocking shoe.	[Gravel]	55	24	10	11								
23.5	718.8	11	16	7		Very dense gray GRAVEL WITH SAND AND SILT (A-2-4), some silt; wet.	[Gravel]	61	16	12	11								
25.0	717.3	18	21	8		Dense gray SILT (A-4b), little fine sand; contains interbedded sand seams; wet.	[Silt]												



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

Depth (ft)	Elev. (ft)	Blows per ft	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 21.0'-25.0', 30.0'-32.0', 40.0'-110.0' Water level at completion: 29.4' (beginning of shift, 7/10/08) 20.5' (includes drilling water)	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP	
									% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
28.5	713.8	10 19 25	15	13	4.5+		Hard gray SANDY SILT (A-4a), "and" fine to coarse sand, trace gravel; contains interbedded sand seams; damp.		9	8	33	34	16	NP	
31.8	710.5	13 23 31	13	14	4.5+		Dense to very dense gray COARSE AND FINE SAND (A-3a), some silt; moist.		28	15	19	25	13		
35	710.5	6 29	14	15A 15B	4.5+		Hard gray SANDY SILT (A-4a), some gravel, little to some fine to coarse sand; damp. @ 31.0'-43.5', difficult drilling. @ 33.5', 5 inches sand heave.								
42.0	700.3	14 23 33	14	17	4.5+		Dense gray COARSE AND FINE SAND (A-3a), little silt; contains silty clay seams; wet.								
44.2	698.1	10 20 29	18	18A 18B			Dense gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; moist.								
47.0	695.3	19 37 50/3	15	19			Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet. @ 48.5', 6 inches sand heave.		21	48	27	4	NP		



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Press / Core	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 21.0'-25.0', 30.0'-32.0', 40.0'-110.0' Water level at completion: 29.4' (beginning of shift, 7/10/08) 20.5' (includes drilling water) FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	Graphic Log	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP LL 40 30 20 10		
										% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
84.0	658.3	31 50/3		26A 26B			4.5+	Stiff gray SILT AND CLAY (A-6a), some to "and" fine to coarse sand, trace to little gravel; damp to moist. Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet.		25	20	18	26	11		50+		
		15 50/4		27						31	48	18	--	2			50+	
		13 26 30	18	28														57 10
100	642.3	41 50/2	8	29														50+

@ 98.5', 1.2 feet sand heave.

Project: FRA-70-8.93

Job No. 0221-1004.01

Location: Sta. 745+02.00, 72.77 ft Rt. of I-70 CL

Date Drilled: 7/7/2008 to 7/8/2008

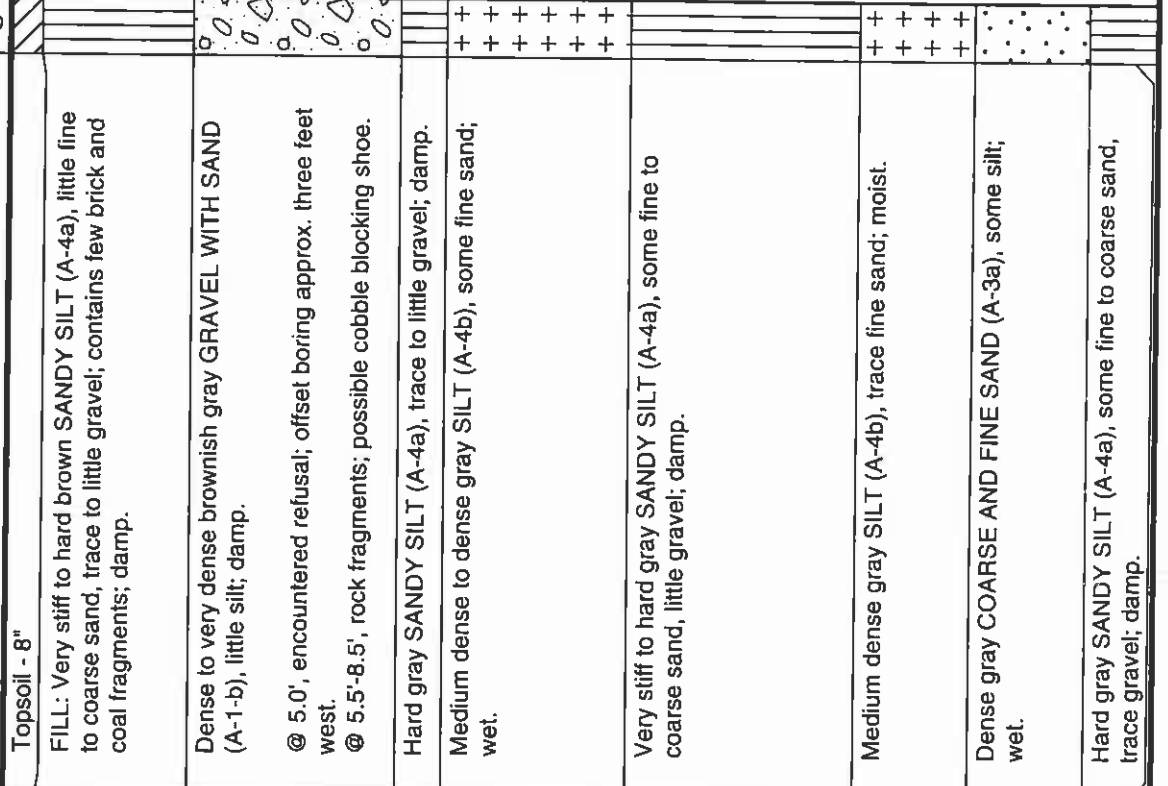
Client: ms consultants

LOG OF: Boring B-031

Depth (ft)	Elev. (ft)	Blows per ft	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
							% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	
0.7	735.6					Topsoil - 8"							
	734.9	2		1	3.75	FILL: Very stiff to hard brown SANDY SILT (A-4a), little fine to coarse sand, trace to little gravel; contains few brick and coal fragments; damp.							
		4	18										
		5		2	4.5+								
		3											
4.0	731.6	4	18			Dense to very dense brownish gray GRAVEL WITH SAND (A-1-b), little silt; damp.	12	5	6	52	25		
		5		3									
		16	5										
		50/5		4		@ 5.0', encountered refusal; offset boring approx. three feet west.	50	20	13	17			
		42				@ 5.5'-8.5', rock fragments; possible cobble blocking shoe.							
		44	1										
		31											
		18											
8.5	727.1	21	1		4.5	Hard gray SANDY SILT (A-4a), trace to little gravel; damp.	15	12	25	32	16		
		25				Medium dense to dense gray SILT (A-4b), some fine sand; wet.							
9.5	726.1	9		6A									
		15	18										
		17		6B									
		4											
		12	18										
		14		7									
13.5	722.1												
		18											
		22	12										
		25		8									
		14											
		48	18										
		49		9									
18.5	717.1												
		11											
		13	18										
		13		10									
21.0	714.6												
		7											
		18	18										
		21		11									
23.5	712.1												
		12											
		18	18										
		21		12									
25	710.6												
		21	18										

WATER OBSERVATIONS:
Water seepage at: 9.5'
Water level at completion: 8.3' (includes drilling water)

FIELD NOTES:
Advanced boring using 3.25" diameter hollowstem augers.



Client: ms consultants

Project: FRA-70-8.93

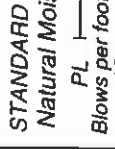
Job No. 0221-1004.01

LOG OF: Boring B-032

Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Date Drilled: 7/18/2008 to 7/15/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 47.0' Water level at completion: 25.7' (includes drilling water)	FIELD NOTES:	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP		
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay	
1.1	750.3	5			4.5+		Asphalt Concrete - 5"	8	7	---	13	29	43		
2.0	749.4	10	9	1	4.5+		Portland Cement Concrete - 3" Aggregate Base - 5"								
5	745.4	6		2	2.0		FILL: Medium dense brown COARSE AND FINE SAND (A-3a), little silt, little gravel; moist.								
6.0	745.4	6		3	2.5		FILL: Very stiff to hard brown SILTY CLAY (A-6b), little to some fine to coarse sand, trace gravel; contains few brick fragments; damp to moist.								
8.5	742.9	4		4	4.5+		Very stiff brown SILT AND CLAY (A-6a), little fine to coarse sand, trace gravel; moist.								
11.0	740.4	7		5	4.5+		Medium dense gray COARSE AND FINE SAND (A-3a), little silt, little gravel; damp. (POSSIBLE FOUNDRY SAND)								
15		12		6	4.5+		Hard gray SILTY CLAY (A-6b), little to some fine to coarse sand, trace to little gravel; damp. @ 11.0'-12.5', brown and gray.	12	8	---	15	38	27		
20		16		7	--		@ 13.0'-35.0', difficult drilling; possible cobbles and boulders.								
		17		8	--		@ 18.5'-20.0', only gravel recovered in auger cuttings sample.								
		50/1		9	--										
		50/3		10	4.5+		Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; damp.	7	14	---	20	37	22		
23.5	727.9	34													
	726.4	17													
		21													



Graphic Log

Client: ms consultants

Project: FRA-70-8.93

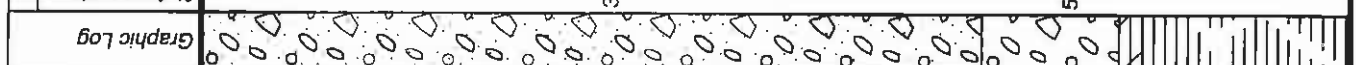
Job No. 0221-1004.01

LOG OF: Boring B-032

Date Drilled: 7/8/2008 to 7/15/2008

Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 47.0' Water level at completion: 25.7' (includes drilling water)	FIELD NOTES:	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP							
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay								
651.4																					
106		37 50/3	15	29			Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet.														
110		27 50/4	10	30																	
115		50/5	5	31			@ 113.5'-113.9', possible cobbles.														
117.0	634.4																				
120.0	631.4	22 50/4	10	32			Very dense gray GRAVEL (A-1-a), "and" fine to coarse sand, trace silt; wet.														
120.5	630.9	50/3	3	33			Severely weathered gray SHALE.														
125	626.4						Shale, dark gray, highly to severely weathered, very weak to weak, laminated, calcareous, friable, fissile, pyritic, jointed, fractured to highly fractured, tight, slightly rough; RQD 5% Loss 53%.														



Client: ms consultants

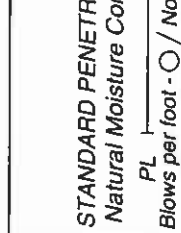
Job No. 0221-1004.01

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

Depth (ft)	Elev. (ft)	Blows per ft	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP										
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay									
0.3	751.5					Topsoil - 3"																	
1.5	751.2	19		1		Brick Fragments																	
3.5	748.0	35	8	2	1.5	FILL: Stiff brown SANDY SILT (A-4a), little fine to coarse sand, little gravel; contains numerous 1/8"-1/4" size brick fragments; moist.																	
6.0	745.5	7	3	3		FILL: Medium dense brown GRAVEL WITH SAND (A-1-b), little silt; contains numerous 1/8"-1/4" size brick fragments; wet.																	
8.5	743.0	5	16	4		Brick Fragments																	
11.0	740.5	2	8	5	1.75	FILL: Stiff brown and gray SANDY SILT (A-4a), some fine to coarse sand, some gravel; contains brick and rock fragments; moist.																	
16.0	735.5	4	10	6	--	FILL: Loose brown GRAVEL WITH SAND (A-1-b), little silt; contains few 1/8"-1/2" size brick fragments; wet.																	
18.5	733.0	6	6	7	4.5+	Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; contains few 1/8"-1/2" size brick fragments; damp.																	
21.0	730.5	2	13	8	4.5+	Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; possible cobbles; wet.																	
23.5	728.0	11	16	9	4.5+	Hard gray SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.																	
25	726.5	13	2	10																			



Client: ms consultants

Job No. 0221-1004.01

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

Depth (ft)	Elev. (ft)	Blows per ft	Recovery	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 5.5', 10.5', 20.5', 25.5', 62.0' Water level at completion: 43.3' (measured inside casing) At beginning of shift: 8/8 28.5. FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, %
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	
28.5	723.0	19	6	11		Medium dense gray GRAVEL (A-1-a), little to some fine to coarse sand, trace silt; contains few brick fragments (likely picked up at higher elevation); wet.	0	74	15	6	5	NP	50+
31.0	720.5	50/5	6	12		Very dense gray GRAVEL WITH SAND (A-1-b), little silt; wet.	0	36	34	15	15	NP	50+
33.5	718.0	6	4	13		Very loose to loose gray GRAVEL (A-1-a), little fine to coarse sand, trace silt; contains few brick fragments (likely picked up at higher elevation); wet.	0	81	12	4	3	NP	50+
38.5	713.0	19	30	14		Very dense gray GRAVEL WITH SAND (A-1-b), trace to little silt; wet.	0	33	30	21	16	NP	50+
41.0	710.5	8	11	15		@ 36.0'-37.5', contains 3/4"x1-1/2" steel fragment - possible conduit fragment (likely picked up at higher elevation); wet.	0						50+
44.0	707.5	16	35	16		Very dense gray SILT (A-4b), trace silt, trace clay; contains occasional sand seams; wet.	+						50+
47.0	704.5	8	9	17		Medium dense gray GRAVEL (A-1-a), little fine to coarse sand; wet.	+	87	9	3	1	NP	50+
		25	30	18		@ 43.5'-44.0', fine sand seam.	0						50+
		30	18	18		Very soft gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; wet.	0						50+
		28	50/5	19		Very dense gray GRAVEL (A-1-a), some fine to coarse sand; wet.	0						50+

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Press / Core	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 5.5', 10.5', 20.5', 25.5', 62.0' Water level at completion: 43.3' (measured inside casing) At beginning of shift: 8/8 28.5, BOH 45.0' FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP
										% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	
87.0	664.5	25 38 50/5	15	25				Very dense gray GRAVEL WITH SAND (A-1-b), some silt; moist.		28	31	9	32		50+
92.0	659.5	22 35 50/5	17	27				Very dense gray SANDY SILT (A-4a), some gravel; moist.							50+
97.0	654.5	35 50/5	9	28				Very dense gray GRAVEL WITH SAND (A-1-b), "and" fine to coarse sand, little silt; wet.							50+
100	651.5	50/5	5	29			3.25	Very stiff gray SANDY SILT (A-4a), some fine to coarse sand, some gravel; possible cobbles; moist.							50+

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	Graphic Log	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP					
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		PL	LL			
	626.5					<p>WATER OBSERVATIONS: Water seepage at: 5.5', 10.5', 20.5', 25.5', 62.0' Water level at completion: 43.3' (measured inside casing) At beginning of shift: 8/8 28.5', 8/9 45.0', Advanced boring using 4.0" diameter flush joint casing.</p> <p>FIELD NOTES: Shale, dark gray, highly to severely weathered, weak, thinly laminated, fissile, calcareous, pyritic, jointed, fractured, tight, slightly rough, RQD 69%, Loss 12%. Contains occasional large (up to 1" diameter) pyritic inclusion. @ 126.7'-127.2', 129.8'-130.4', 131.1'-131.5', high angle fractures. @ 130.0' -130.3', qu = 3978 psi @ 133.5'-133.7', encountered thin limestone bed.</p>													
135.5	616.0	Core Rec 60" 60"		RQD R-3 57%															
		Core Rec 60" 60"		RQD R-4 85%															
140																			
145																			
150																			

Bottom of Boring - 135.5'

Client: ms consultants

Project: FRA-70-8.93

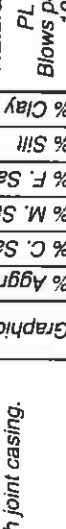
Job No. 0221-1004.01

LOG OF: Boring B-037

Location: Sta. 756+80.53, 69.18 ft Lt. of I-70 CL

Date Drilled: 7/21/2008 to 7/24/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Press / Core	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 21.0', 25.4', 33.0', 40.5' Water level at completion: 25.6' (prior to coring) 51.6' (includes drilling water) FIELD NOTES: Advanced boring using 3.0" diameter flush joint casing.	DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % ● Blows per foot - ○ / Non-Plastic - NP			
											% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay		
1.8	747.3	5					--	Asphalt Concrete - 6.5" Portland Cement Concrete - 9.5" Aggregate Base - 5"											
5.5	740.6	9	0	1			1.75	Stiff brown and gray SANDY SILT (A-4a), some fine to coarse sand, some gravel; increased moisture content due to pavement coring; moist to wet.				29	15	17	24	15			
8.5	740.6	6	1	3			--												
10.0	740.6	8	2	4			--	Very stiff gray SILT AND CLAY (A-6a), little to some fine to coarse sand, little gravel; damp to moist.											
13.5	735.6	4	18	5			3.75												
15.0	735.6	10	2	6			4.25	Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.											
20.0	729.1	7	18	8			4.5+					8	11	22	40	19			
23.0	726.1	10	5	9			0.75	Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, little gravel; damp. @ 20.0'-21.5'; stiff, wet; possible sand and gravel seam.											
25.0	724.1	19	18	10			4.5+	Hard gray SANDY SILT (A-4a), trace to little gravel; contains sand seams; damp.											
25.0	724.1	48	2	11			4.5+												



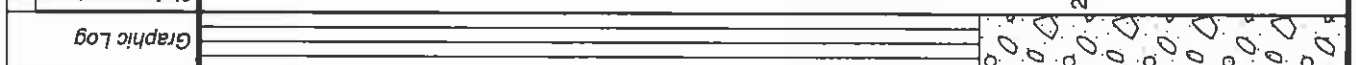
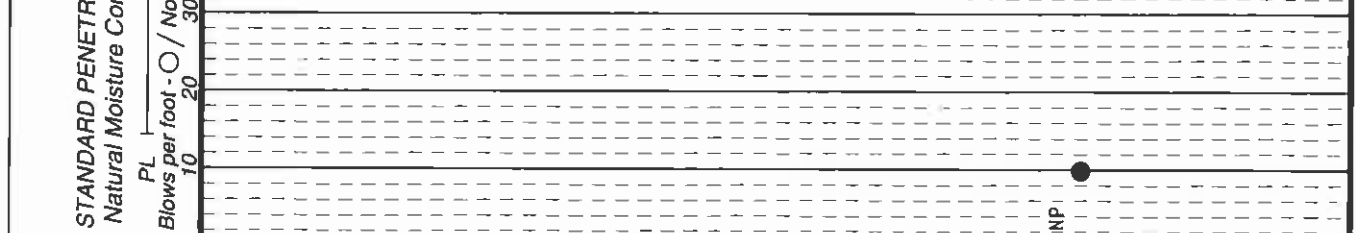
Client: ms consultants Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-043 Location: Sta. 765+56.96, 96.30 ft Rt. of I-70 CL

Date Drilled: 7/16/2008 to 7/20/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ●	
							% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		PL Blows per foot - ○
718.1														
42.0	701.1													
30		20 42 50/5	17	13	4.5+	Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.								50+
		24 34 45	18	14	4.5+									
35		11 30 40	18	15	4.5+	@ 38.5', becomes wet.								74
40		13 28 45	10	16	--									
45		17 34 38	18	17		Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet.								76
50	693.1	13 31 39	18	18							22	42	27	NP



WATER OBSERVATIONS:
 Water seepage at: 37.0', 77.0'
 Water level at completion: 14.4' (includes drilling water)

FIELD NOTES:
 Advanced boring using 3.25" diameter hollowstem augers.

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-044

Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL

Date Drilled: 6/16/2008 to 6/19/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Dive	Press / Core	Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: 13.5'-14.2', 23.5'-25.0' Water level at completion: 19.1' (beginning of shift, 9/17/08) 20.8' (includes drilling water)	FIELD NOTES: Advanced boring using 4.25" diameter hollowstem augers to 47.0'; 4.0" casing from 47.0' to 115.5'. DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP			
											% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay		
1.3	764.6	10					--		Asphalt Concrete Pavement - 2"										
3.5	763.3	9	8	1			--		Portland Cement Concrete - 4"										
5.5	761.1	8	8	2			2.5		Aggregate Base - 10"										
5.5	759.1	5	11	3			4.0		POSSIBLE FILL: Stiff to very stiff brown SILTY CLAY (A-6b), little to some fine to coarse sand, trace gravel; moist.										
11.0	753.6	12	18	4			2.0		POSSIBLE FILL: Very stiff mottled olive-brown and gray CLAY (A-7-6), little fine to coarse sand, trace gravel; moist.										
13.0	751.6	15	18	5					Very stiff brown SANDY SILT (A-4a), trace gravel, some clay; damp.										
14.0	750.6	13	17	6					@ 11.0'-16.0', encountered very difficult drilling, dense till, possible cobbles. Recovered rock fragments in split spoon.										
15.0	749.6	18	18	7A					Dense brown GRAVEL WITH SAND AND SILT (A-2-4); damp.										
18.0	746.6	21	16	7B					Stiff gray SILT AND CLAY (A-6a), some fine to coarse sand, little to some gravel; damp to moist.										
		20	16	8			4.0		Very dense brown GRAVEL (A-1-a), little silty clay; moist to wet.										
		14	16	9			3.5		@ 14.2', refusal of splitspoon.										
		17	18	10			2.75		Very stiff to hard gray SANDY SILT (A-4a), little to some clay, trace to little gravel; damp.										
		17	18	11			--		Very stiff to hard gray SILT AND CLAY (A-6a), little to some clay, little gravel; damp to moist.										
25	739.6	35	18						@ 20.5', moist to wet.										

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-044

Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL

Date Drilled: 6/16/2008 to 6/19/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Drive	Press / Core	Hand Penetro-meter (tsf)	WATER OBSERVATIONS: Water seepage at: 13.5'-14.2'; 23.5'-25.0' Water level at completion: 19.1' (beginning of shift, 9/17/08) 20.8' (includes drilling water) FIELD NOTES: Advanced boring using 4.25" diameter hollowstem augers to 47.0'; 4.0" casing from 47.0' to 115.5'. DESCRIPTION	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP	
										% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
28.0	739.6	12		12			3.0	Very stiff to hard gray SILT AND CLAY (A-6a), little to some clay, little gravel; damp. @ 26.0', contains few thin (less than 1") fine to medium grained sand seams.		18	12	---	20	27	23	
30	736.6	13	17	13			4.0	Very stiff to hard gray SANDY SILT (A-4a), little to some clay, little gravel; damp.								
35		35	4	14			--	@ 33.9', encountered possible cobble or boulder.								
39.6	725.0	14		15A			3.5	@ 36.0', pulled augers to change lead. Lead auger bit destroyed. Could not advance further with 500-600 psi down pressure. Changed tools to drill with casing.								
40		37	18	15B			--	Very dense gray COARSE AND FINE SAND (A-3a), little silty clay, trace gravel; wet.								
42.0	722.6	48						Hard gray SANDY SILT (A-4a), "and" fine to coarse sand, trace gravel; damp.								
45		21	17	16			4.5+	Very dense gray GRAVEL WITH SAND (A-1-b), some silty clay; wet.								
47.0	717.6	42														
50	714.6	10	6	17												

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-045

Location: Sta. 768+75.91, 91.18 ft. Rt. of I-70 CL

Date Drilled: 7/17/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	DESCRIPTION	GRADATION						STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP								
							% Aggregate	% C Sand	% M. Sand	% F. Sand	% Silt	% Clay									
1.1	745.8																				
	744.7					Asphalt Concrete - 8" Aggregate Base - 5"															
		7		1	4.5+	Hard gray SANDY SILT (A-4a), little clay, trace to little gravel; damp. @ 5.6', sand and gravel seam.															
		9																			
		9	14																		
5		7		2	4.5+																
		9																			
		9	10																		
		3		3	4.0																
		7																			
		11		4	4.5+																
		12																			
		15																			
		19		5	4.5+																
8.0	737.8	13				Hard gray SILT AND CLAY (A-6a), some to "and" fine to coarse sand, trace gravel; damp. @ 8'0"-9'5", contains sand seams.															
		16																			
		17																			
		18		6	4.5+																
		9																			
		11																			
		16																			
		27		7	4.5+																
		27																			
		10																			
		24																			
15.0	730.8	23		8	4.5+																
						Bottom of Boring - 15.0'															