## **ROADWAY EXPLORATION**

Intersection Improvements
GEA/LAK-44-18.45/00.00, PID 114163
State Route 44

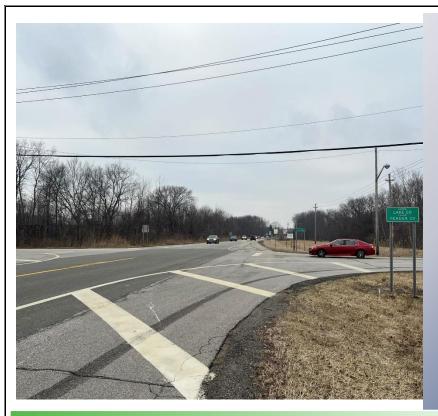
Chardon & Concord Townships, Geauga & Lake Counties, Ohio



Submitted to GPD Group FINAL REPORT - Date August 2024







## Intersection Improvements GEA/LAK-44-18.45/00.00, PID 114163

Chardon & Concord Townships, Geauga & Lake Counties, Ohio

## **Final Report - Roadway Exploration**

GPD Group Cleveland, Ohio

August 12, 2024

TTL Project No. 2279801



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August 12, 2024

TTL Project No. 2279801

Mr. David Neumeyer, P.E. GPD Group 1228 Euclid Avenue, Suite 320 Cleveland, Ohio 44115

Final Report
Roadway Exploration
GEA/LAK-44-18.45/00.00, PID 114163
Intersection Improvements
Chardon & Concord Townships, Geauga & Lake Counties, Ohio

Dear Mr. Neumeyer:

Following is the report of our roadway exploration performed by TTL Associates, Inc. (TTL) for the referenced project. This study was performed in accordance with TTL Proposal No. 2279801R, dated August 19, 2022, and was authorized by GPD Group via a subconsultant agreement for professional services, dated September 30, 2022, which was fully-executed on October 12, 2022. Email correspondence provided on November 22, 2022 from ODOT District 12 indicated that Borings B-059, B-062, and B-064 should be eliminated from the scope of work and Boring B-060 should be moved to the north side of Girdled Road, between Auburn Road and SR44.

A draft version of this report was provided to you on March 8, 2023. ODOT comments were provided on September 28, 2023 regarding the boring logs and on April 16, 2024 regarding the draft report. We understand that the project now consists of widening at intersections and milling/resurfacing for the portions between the intersections. This final report contains the results of our study, our engineering interpretation of the results with respect to the project characteristics, our recommendations for design and construction of pavements as well as potential modifications to subgrade soils for the widening portion of the project. Logs of borings performed in milling/resurfacing areas are provided separate from those associated with widening areas, as they are still pertinent with respect to the existing pavement cross-sections.

Should you have any questions regarding this report or require additional information, please contact our office.

Sincerely,

TTL Associates, Inc.

Christopher P. Iott, P.E.

Chief Geotechnical Engineer

Curtis E. Roupe, P.E.

Vice President

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# FINAL REPORT ROADWAY EXPLORATION GEA/LAK-44-18.45/00.00, PID 114163 INTERSECTION IMPROVEMENTS CHARDON & CONCORD TOWNSHIPS, GEAUGA & LAKE COUNTIES, OHIO

#### **FOR**

#### GPD GROUP 1228 EUCLID AVENUE, SUITE 320 CLEVELAND, OHIO 44115

**SUBMITTED** 

**AUGUST 12, 2024 TTL PROJECT NO. 2279801** 

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#### **EXECUTIVE SUMMARY**

This roadway exploration report has been prepared for the intersection improvements along State Route 44 (SR44), from the Chardon North Corporation line in Chardon Township, Geauga County, north to the 4-lane section, south of Capital Parkway, in Concord Township, Lake County. Intersections within the project area planned for improvements include Hosford Road, Clark Road, Colburn Road, and Girdled Road. This exploration included 61 test borings, 20 of which included a pavement core, as well as five pavement core only locations, for the evaluation of existing pavement sections and subgrade conditions in areas of proposed roadway intersection improvements. The project now includes only widening at the intersections, and milling/resurfacing in other project areas. Subgrade evaluations were performed in accordance with ODOT GB-1 "Plan Subgrades" (Now Geotechnical Design Manual Section 600) for the borings in the widening areas. A summary of the conclusions and recommendations of this study are as follows:

- 1. The cores and majority of the borings were performed through existing roadway pavements. The pavements consisted of asphalt at the surface. Approximately 60 percent of the cores/borings encountered concrete underlying the asphalt. Sand base was present underlying many of the composite asphalt/concrete sections. Some borings in proposed widening areas were performed beyond the extent of existing roadway pavements, and encountered aggregate shoulder material or topsoil.
- 2. Existing fill materials were encountered in eight of the 61 borings, representing approximately 13 percent of the borings performed for this exploration. Fill materials were indicated where non-soil materials such as crushed stone or asphalt fragments were present in the soils. Granular fill materials were encountered in four borings, and cohesive fill materials were encountered in four borings.
- 3. The subsoils encountered immediately underlying the surface materials predominantly consisted of cohesive soils. Cohesive subgrade soils were encountered in approximately 75 percent of the borings performed for this exploration. The cohesive soils were generally classified as A-4a or A-6a, although zones of A-6b subgrade soils were also encountered.
- 4. Native granular subgrade soils were encountered in 11 borings, representing approximately 18 percent of the test borings performed for this exploration. The native granular subgrade soils were classified as A-1-b and A-3a. It should be noted that encountered granular soils were more prevalent during this exploration than what was identified in the historic borings performed for the original construction of SR44 along this alignment. As such, some of the encountered granular soils may be fill materials placed to achieve design subgrades. However, non-soil materials were not observed in the recovered samples. Therefore, they have not been identified as fill.
- 5. Based on the limited data available, such as the soil characteristics and the groundwater conditions encountered in the borings, it is our opinion that the "normal" groundwater level is anticipated to generally be encountered at depths of approximately 5 feet or greater below existing grades. It should be noted that "perched" water may be encountered in the fill materials



or granular soils that are underlain by relatively impermeable native cohesive soils, or at the soil/bedrock interface. If construction does not occur during a particularly wet period, adequate control of groundwater seepage into excavations extending only a few feet below the "normal" groundwater level should be achievable by minor dewatering systems, such as pumping from prepared sumps. If saturated granular soils are encountered, they may require installation of well points in addition to pumping from prepared sumps.

- 6. Based on the ODOT "Subgrade Analysis" worksheet, 15 of the 25 borings in intersection areas of planned widening contained subgrade soils within the upper profile which indicated subgrade modification is likely to be required. Three (3) of these borings indicated planned subgrade modification consisting of reworking granular soils in-place. Based on the GB-1/ODOT Geotechnical Design Manual Section 600 analysis results, subgrade modification may consist of global chemical stabilization using cement to a depth of 14 inches, or over-excavation and replacement with new granular engineered fill. With widening planned for widths generally 12 feet or less, it is not expected that global chemical stabilization equipment could readily access and treat the subgrades. As such, it is planned that subgrade modification for this project include over-excavation and replacement with new granular engineered fill.
- 7. The ODOT "Subgrade Analysis" worksheet resulted in a CBR value of 7 percent for the project site. It should be noted that the CBR determination by the Subgrade Analysis spreadsheet is based on an average Group Index (GI) of all the evaluated samples. It should be noted that group indices for the tested samples ranging from 9 to 16, which would correlate with lower CBR values than that determined based on the average GI value of 7, were generally encountered in borings that were indicated by the Subgrade Analysis spreadsheet for subgrade modification, or were present at depths of at least 1½ feet below the anticipated top of subgrade elevation. As such, it does not appear to be unconservative to use the Subgrade Analysis worksheet design CBR value of 7 percent based on the average design value calculations.

This executive summary highlights our evaluations and recommendations and should only be utilized in conjunction with the accompanying report, including the detailed findings, analysis and recommendations, and qualifications presented herein.



#### TABLE OF CONTENTS

		Page No.
1.0	INTRODUCTION	1
1.1	Purpose and Scope of Exploration	1
1.2	Proposed Construction	2
2.0	GEOLOGY AND OBSERVATIONS OF THE PROJECT	3
2.1	General Geology and Hydrogeology	3
2.2	Site Reconnaissance	4
3.0	EXPLORATION	5
3.1	Historic Borings	5
3.2	Project Exploration Program	6
3.3	Boring Methods	8
3.4	Laboratory Testing Program	9
4.0	FINDINGS	10
4.1	General Site Conditions	10
4.2	General Soil Conditions	13
4.3	Groundwater Conditions	16
4.4	Remedial Measures	17
5.0	ANALYSES AND RECOMMENDATIONS	18
5.1	GB-1 "Plan Subgrades" Evaluation	18
5.2	Flexible (Asphalt) Pavement Design	21
5.3	Site and Subgrade Preparation	22
5.4	Groundwater Control	23
5.5	Excavations and Slopes	23
5.6	Fill	24
6.0	OUALIFICATION OF RECOMMENDATIONS	26



#### **TABLE OF CONTENTS (Cont.)**

#### **PLATES**

1.0 Site Location Map

2.1 through 2.3 Test Boring Location Plans (South, Middle, North)
3.1 through 3.4 Test Boring Location Plans (Zoom In to Intersections)

#### FIGURES - INTERSECTIONS BORINGS/CORES

Logs of Test Borings

Legend Key

Grain Size Distribution

Photographic Logs of Selected Pavement Cores

#### **APPENDICES**

Appendix A: Engineering Calculations (including Subgrade Analysis Spreadsheet)

Appendix B: Non-Intersection Logs of Test Borings, Grain Size Distribution, and

Photographic Logs of Selected Pavement Cores

Appendix C: Geotechnical Engineering Design Checklists

Appendix D: Historic Borings

#### 1.0 INTRODUCTION

This roadway exploration report has been prepared for the intersection improvements along State Route 44 (SR44), from the Chardon North Corporation line in Chardon Township, Geauga County, north to the 4-lane section, south of Capital Parkway, in Concord Township, Lake County. Intersections within the project area planned for improvements include Hosford Road, Clark Road, Colburn Road, and Girdled Road. Milling/resurfacing is planned for the other project areas. The general project area is shown on the attached Site Location Map (Plate 1.0).

This study was performed in accordance with TTL Proposal No. 2279801R, dated August 19, 2022, and was authorized by GPD Group via a subconsultant agreement for professional services, dated September 30, 2022, which was fully-executed on October 12, 2022. Email correspondence provided on November 22, 2022 from ODOT District 12 indicated that Borings B-059, B-062, and B-064 should be eliminated from the scope of work and Boring B-060 should be moved to the north side of Girdled Road, between Auburn Road and SR44.

#### 1.1 Purpose and Scope of Exploration

The purpose of this exploration was to evaluate the pavement conditions, subsurface conditions and laboratory data relative to the design and construction of pavements for the referenced project. To accomplish this, TTL performed 61 test borings, 20 of which included a pavement core, as well as five pavement core only locations, laboratory soil testing, a geotechnical engineering evaluation of the test results, and review of available geologic and soils data for the project area.

The project now includes widening at the intersections and milling/resurfacing for the other project areas. As such, data for the widening areas are incorporated as figures attached to this report. The data for the borings and cores performed in areas that are now just milling/resurfacing have been included in Appendix B.

This report summarizes our understanding of the proposed construction, describes the investigative and testing procedures utilized to evaluate the subsurface conditions at the site, and presents our findings from the field and laboratory testing. This report also presents our evaluations and conclusions in accordance with ODOT GB-1 "Plan Subgrades" (Now Geotechnical Design Manual Section 600) and provides our design and construction recommendations for pavements.



#### This report includes:

- A description of the existing surface materials, subsurface soils, and groundwater conditions encountered in the pavement cores and borings.
- Design recommendations for pavements.
- Recommendations concerning soil and groundwater-related construction procedures such as subgrade preparation in accordance with ODOT GB-1 criteria, earthwork, pavement construction, and related field testing.

Appendix C includes pertinent ODOT Geotechnical Engineering Design Checklists that apply to the scope of this report. This exploration did not include an environmental assessment of the surface or subsurface materials at the site.

#### 1.2 Proposed Construction

The intersection improvements include widening in some areas, generally on the order of 12 feet or less in width. Milling/resurfacing is planned in other project areas.

Final roadway grades will approximate existing roadway grades and consist of asphalt pavements. New pavement widening areas are planned to include 9½ inches of asphalt underlain by 6 inches of aggregate base, for a total cross-section of 15½ inches (approximately 1.3 feet). For consideration of milling/resurfacing, existing pavement cross-sections [asphalt, concrete (where encountered), and aggregate base or sand base combined] encountered in the borings performed for this exploration varied from 7 to 28 inches in thickness, with an average of approximately 18 inches considering all borings/cores performed in existing roadways for this exploration.



#### 2.0 GEOLOGY AND OBSERVATIONS OF THE PROJECT

#### 2.1 General Geology and Hydrogeology

Published geologic maps from the Ohio Department of Natural Resources (ODNR) indicate that the project corridor is located in the Killbuck-Glaciated Pittsburgh Plateau Region of the Glaciated Allegheny Plateaus Section. Within this region, thin to thick Wisconsinan-age clay to loam till is present overlying Mississippian- and Pennsylvanian-age shales, sandstones, conglomerates and coals.

The USDA Natural Resource Conservation Service (NRCS) Web Soil Survey indicates that soils in the project area are predominantly mapped as various silt loams. The predominant soil type consisted of Mahoning silt loam, which constituted approximately one-third of the project area, followed by Ellsworth silt loam, which constituted approximately 15 percent of the project area, and Platea silt loam, which constituted approximately 10 percent of the project area. These soils consist of glacial till formed on till plains.

On the "Geologic Map of Ohio," the project corridor is mapped as bedrock predominantly consisting of Mississippian-age Waverly shales, sandstone, and limestone, but also includes Pennsylvanian-age Pottsville and Alleghany coal, sandstone, shales, and limestone near the southern extent, as well as Devonian-age Olentangy and Ohio shales near the northern extent. Rock coring was not performed as part of this exploration. However, where weathered bedrock samples were obtained using a split-spoon sampler, they were visually identified as sandstone (some of which contained trace shale). Based on water well logs, bedrock depths in various portions of the project corridor are summarized in the following table, along with bedrock depths in the borings performed for this exploration.

Table 2.1. Bedrock Depth in Project Area					
General Area of	General Range of Depth to Bedrock (ft)				
Intersection with SR44	From Water Well Logs	Encountered in the Project Test Borings			
Mentor Road		B-001 through B-004: 3 to 7 ft			
1.1011tol 1toug	15 to 25 ft	(Possibly shallower due to excavation			
(Near South Extent)		performed to achieve SR44 grade)			
Hosford Road	20 to 60 ft	N.E.			
Clark Daad	20 to 75 ft	B-025: 6½ ft; B-034: 6 ft			
Clark Road		N.E. in other borings.			
Colburn Road	5 to 25 ft	B-045: 7 ft			
Colburn Road	3 to 23 ft	N.E. in other borings.			
Cirdled Bood	4 to 8 ft	B-051: 4 ft, B-052: 4½ ft, B-063: 4 ft			
Girdled Road	4 10 8 11	N.E. in other borings.			
Capital Parkway	5 to 10 ft	NE			
(Near North Extent)	5 to 10 ft	N.E.			

N.E. – Not Encountered (Present at depths deeper than 7.5 to 10 feet termination depths).



Based on the ODNR "Mines of Ohio" interactive map website, mining is not indicated in the project area. A historic quarry is indicated along the northeast side of Mentor Road, approximately ¾ mile southwest of the intersection of SR44 with Hosford Road. Additionally, a surface mind for Keeney Sand & Stone, Inc. is identified approximately 4¼ miles east of the intersection of SR44 with Girdled Road.

Based on the ODNR "Ohio Karst Areas" map, the project corridor is indicated in an area not known to contain karst features.

#### 2.2 Site Reconnaissance

TTL performed boring layout on December 1, 2022 to facilitate drilling operations in December 2022. TTL then performed a more formal site reconnaissance on March 2, 2023. The site is located in a predominantly rural area abutting the backyards of scattered residential properties. Commercial area with many retail and lodging developments were located along the northern end of the project corridor. The Maple Highland walking trail was located along the southern portion of SR44 (starting south of Boring B-023) and its centerline was approximately 15 to 20 feet east of the edge of pavement.

The pavements along SR44 were observed to be in generally fair to good condition. Signs of distress were not noticeable throughout much of the pavement areas. However, there were areas of observed longitudinal cracking down the center of the pavement, along with minor localized transverse cracking (generally spanning from edge to edge of pavement).

The pavement along Girdled Road, east of SR44, appeared to be recently resurfaced and signs of distress were not noticeable. However, in this general portion of the project area, patched areas were observed in SR44 and Crile Road.

Within the project area, SR44 appeared to be located within a generally hilly area, with scattered riparian topography and associated creeks or perennial streams. The roadway surface of SR44 was observed to be generally approximately 3 to 6 feet above surrounding grades. Multiple existing culvert pipes were observed to cross beneath the roadway. A swale was observed to extend along SR44 in the portion where the walking trail was present. The swale was observed to be 1 to 2 feet deep and exhibited ponded water in some areas, albeit after a period of rainfall the prior week.



#### 3.0 EXPLORATION

#### 3.1 <u>Historic Borings</u>

Review of the ODOT Transportation Information Mapping System (TIMS) for the project area indicated historic geotechnical information was available for the original construction of the current SR44 alignment via Project GEA/LAK-44-(18.30)(0.00). The geotechnical work for this project was performed during 1961. The geotechnical Soil Profile for this historic project is included in Appendix C of this report. It should be noted that the available soil profile from the TIMS only includes sheets 1 through 20 of 23. Included in the set are a cover sheet, laboratory test results sheets, plan-and-profile sheets, and cross-section sheets. The sheets include cross-sections from the southern project area north to Sta. 523+00, which is located approximately 2,000 feet south of Hosford Road.

It should be noted that the historic borings did not include Standard Penetration Test (SPT) results. Therefore, only a summary of the geotechnical information from the historic Soil Profile is provided in this section, and the Soil Profile is included in Appendix C. However, the historic borings are not included in evaluations, recommendations, or the Plates associated with this report, and will not be included in the Geotechnical Profile sheets for this project.

Based on the historic Soil Profile, the southern portion of the project area originally included rolling topography, which required alternating cut and fill to achieve current SR44 grades. Maximum cut and fill in the southern portion of the current project area was on the order of 25 feet. From approximately 1,600 feet south of Clark Road extending to the northern current project extent, maximum cut and fill was on the order of 10 feet or less.

The soils encountered in the historic borings predominantly consisted of cohesive soils classified as ODOT A-4a, A-6a, and A-6b soils, with occasional A-4b and A-7-6 soils. Approximately 8 percent of the tested samples consisted of granular soils, classified in the A-1, A-3, and A-2 series. Sandstone and shale bedrock were indicated in borings performed near the southern extent of the current project area, in the general area of the intersections with Colburn Road and Girdled Road, as well as near the northern extent of the current project area. It should be noted that hand auger refusal was indicated for many historic borings, but some of these borings may have encountered hand auger refusal due to difficult advancement in soils and not necessarily due to bedrock.

We have assumed that the information provided in the historic borings was accurate and correct, at the time of the respective exploration, but cannot guarantee as such. Additionally,



subgrade soil conditions may have changed or may have been modified due to construction performed following completion of the historic subsurface explorations.

#### 3.2 Project Exploration Program

This exploration included 61 test borings, 20 of which included pavement cores, as well as five (5) pavement core only locations. The cores and borings were performed by TTL during the period from December 12 through 22, 2022.

Based on the current project approach of widening at the intersections and milling/resurfacing in the other project areas, 25 test borings, 8 of which included pavement cores, as well as the five pavement core only locations are in intersection areas where widening is planned. The data for these borings and cores are included as figures in this report. The data for the remaining borings is included in Appendix B. The existing pavement cross-sections for these borings/cores were considered pertinent to the planned milling/resurfacing portion of the project.

The cores/borings were generally extended through the existing State Route 44 (SR44) pavements on alternating northbound and southbound sides of the road, but also included three borings performed along Girdled Road. A few of the borings performed north of Girdled Road were performed outside of the pavement extents due to potential for widening in this area. Additionally, Boring B-016-0-22 was performed northeast of the intersection of SR44 and Hosford Road for a potential roundabout in that area. Although the roundabout is no longer planned, widening will be performed at this intersection. Test borings were designated as Borings B-001-0-22 through B-064-0-22, but Borings B-059-0-22, B-062-0-22, and B-064-0-22 were eliminated from the scope of work by ODOT District 12.

The borings that were considered for intersection widening for the project are summarized in the following table.

Table 3.2.A. Intersection Areas Borings							
Boring Number	Location		Boring Number	Location		Boring Number	Location
B-014	SR44		B-032	SR44		B-050	SR44
B-015	SR44		B-042	SR44		B-051	SR44
B-016	SR44		B-043	SR44		B-052	SR44
B-017	SR44		B-044	SR44		B-053	SR44
B-018	SR44		B-045	SR44		B-060	Girdled Rd
B-028	SR44		B-046	SR44		B-061	Girdled Rd
B-029	SR44		B-047	SR44		B-063	Girdled Rd
B-030	SR44		B-048	SR44			
B-031	SR44		B-049	SR44			

Designations and locations for exploration consisting of only coring the pavement and determination of the presence/thickness of underlying base materials (all of which were in intersection areas planned for widening) are summarized in the following table.

Table 3.2.B. Pavement Core Only Locations				
Core Number	Location			
X-016-1	Westbound Hosford Road, east of SR44.			
X-016-2	Eastbound Hosford Road, west of SR44.			
X-030-1	Eastbound Clark Road, west of SR44.			
X-044-1	Eastbound Colburn Road, west of SR44.			
X-050-1	Northbound SR44, offset west of B-050 performed beyond roadway for possible widening.			

The cores and borings are fully designated as in accordance with ODOT protocol, however the "-0-21" or "-21" portion of the nomenclature is generally omitted for ease of identification in the discussions within this report. The borings were located in the field by TTL in general accordance with plans provided with the proposal for this project. The approximate locations of the cores and borings are shown on the Test Boring Location Plans (Plates 2.1 through 2.3 for the general southern, middle, and northern portions of the project area. Plates 3.1 through 3.4 for zoom in views at the major intersections with SR44).

Stationing, offsets, coordinates and ground surface elevations at the boring locations were obtained based on input of the boring locations into Open Roads Design (ORD). This data is provided on the logs of test borings.

In accordance with the ODOT Specifications for Geotechnical Explorations (SGE), the borings were generally performed as ODOT Type A borings to a depth of 6 feet below top of subgrade, based on subgrade anticipated  $1\frac{1}{2}$  feet below existing grades. Selected borings in areas of new roadway alignment were performed as ODOT Type B borings, extended to termination at a depth of 10 feet. Where auger refusal was encountered, or split-spoon refusal occurred during the final planned sample interval, the boring was terminated.

Experience indicates that the actual subsoil conditions at a site could vary from those generalized on the basis of test borings made at specific locations, especially at previously developed sites such as this site. Therefore, it is essential that a geotechnical engineer be retained to provide soil engineering services during the site preparation and pavement construction phases of the proposed project. This is to observe compliance with the design concepts, specifications, and recommendations, and to allow design changes in the event subsurface conditions differ from those anticipated prior to the start of construction.



#### 3.3 Boring Methods

Pavement cores were obtained using a nominal 4-inch diameter core barrel. Photographic logs of the cores obtained at core-only locations, as well as selected pavement cores from test boring locations, are attached to this report.

The borings were performed with a CME 75 truck-mounted drill rig utilizing 3½-inch diameter solid-stem augers. For ODOT Type A borings, samples were generally obtained continuously using 18-inch split-spoon (SS) sample drives. For ODOT Type B borings, samples were obtained at 2½-foot intervals using 18-inch split-spoon (SS) sample drives. The samples were sealed in jars and transported to our laboratory for further classification and testing.

Split-spoon soil samples were obtained by the Standard Penetration Test Method (ASTM D 1586). The Standard Penetration Test (SPT) consists of driving a 2-inch outside diameter split-spoon sampler into the soil with a 140-pound weight falling freely through a distance of 30 inches. The sampler was driven in three successive 6-inch increments, with the number of blows per increment being recorded. The number of blows per increment was recorded at each depth interval, and these data are presented under the "SPT" column on the Logs of Test Borings attached to this report. The sum of the number of blows required to advance the sampler the second and third 6-inch increments is termed the Standard Penetration Resistance, or N<sub>m</sub>-value, and is typically reported in blows per foot (bpf). The N<sub>m</sub>-values were corrected to an equivalent rod energy ratio of 60 percent, N<sub>60</sub>. The hammer/rod energy ratio for the truckmounted CME 75 drill rig was 66 percent, and was last calibrated on March 15, 2021. The N<sub>60</sub>-values are presented on the attached Logs of Test Borings.

Pavement and soil conditions encountered in the test borings are presented in the Logs of Test Borings, along with information related to sample data, SPT results, water conditions observed in the borings, and laboratory test data. In conjunction with published data and typical correlations, the  $N_{60}$ -values can be evaluated as a measure of soil compactness/consistency as well as shear strength.

Field and laboratory data were incorporated into gINT<sup>TM</sup> software for presentation purposes. It should be noted that these logs have been prepared on the basis of laboratory classification and testing as well as field logs of the encountered soils.



#### 3.4 <u>Laboratory Testing Program</u>

All recovered samples of the subsoils were also tested in our laboratory for moisture content (ASTM D 2216). Unconfined compressive strength estimates were obtained for the intact cohesive samples using a calibrated hand penetrometer. It should be noted that many of the recovered cohesive samples were not intact, so these samples do not include a hand penetrometer reading on the logs of test borings. These test results are presented on the Logs of Test Borings.

Laboratory testing was performed in accordance with GB-1 "Plan Subgrades" (Now ODOT Geotechnical Design Manual Section 600) criteria, including mechanical soil classification consisting of an Atterberg limits test (ASTM D 4318) and a particle size analysis (ASTM D 6913 and D 7928) for at least two samples from each boring within 6 feet of the proposed subgrade. These test results are presented on the Logs of Test Borings and Grain Size Distribution sheets.

Compactness descriptions have been provided in the logs of test borings for samples tested or visually classified as being non-plastic, including non-plastic A-4a soils. Consistency (stiffness) descriptions have been provided in the logs of test borings for cohesive soils, as well as plastic A-4a soils (those with a plasticity index of 1 or greater).

Sulfate content determinations (ODOT Supplement 1122) were generally performed on one sample from each boring, generally within 3 feet of the proposed subgrade. These test results are presented on the Logs of Test Borings. During the initial laboratory testing program, TTL assigned varying SS-1 and SS-2 samples within 3 feet of anticipated subgrade, to evaluate if there were differing conditions within the upper 3 feet and to include data should some grades get lowered or pavement cross-sections result in deeper top of subgrade elevations. We understand that it is ODOT District 12 preference to test SS-1 samples. Additional tests have been performed to incorporate SS-1 samples that were not previously tested at the time of the draft report submittal.



#### 4.0 FINDINGS

#### **4.1** General Site Conditions

The cores and majority of the borings were performed through existing roadway pavements. The pavements consisted of asphalt at the surface. Approximately 60 percent of the cores/borings encountered concrete underlying the asphalt. Sand base was present underlying many of the composite asphalt/concrete sections. Some borings in proposed widening areas were performed beyond the extent of existing roadway pavements, and encountered aggregate shoulder material or topsoil. The surface materials encountered in the cores and borings are summarized in Table 4.1 below. Borings performed in areas of intersections where widening is planned are highlighted in yellow. Photographic logs of selected pavement cores are also attached to this report.

	Table 4.1. Surface Materials Summary						
Boring/Core		Pavemen	t Component Thick	ness (in)			
Number	Asphalt	Concrete	Aggregate Base	Sand Base	Total	Notes	
B-001	6	8	-	-	14		
B-002	6	12	-	-	18		
B-003	9	-	5	-	14		
B-004	7.5	-	6.25	-	13.75		
B-005	6.5	11.5	-	-	18		
B-006	7	11	-	-	18		
B-007	7.5	-	-	7	14.5		
B-008	10.5	-	6.5	-	17		
B-009	6	12	-	6	24		
B-010	6.5	8	-	2	16.5		
B-011	9.5	-	8.5	7	25		
B-012	10	-	5.5	-	15.5		
B-013	6.5	8.5	5	-	20		
B-014	6.5	7.5	-	5	19		
B-015	9.5	-	7.5	-	17		
B-016	-	-	-	-	-	3 inches topsoil, Hosford Rd	
X-016-1	6.25	8	_	5	19.25	Westbound Hosford Road,	
X-010-1	0.23	0		J	13.23	east of SR44.	
X-016-2	4.25	9.25	-	5	18.5	Eastbound Hosford Road, west of SR44.	
B-017	7	-	-	-	7	Underlain by approx. 2-1/2 ft gravel. May be base material and possible backfill to achieve subgrade elevation.	



	Table 4.1. Surface Materials Summary						
Boring/Core		Pavemer	nt Component Thick	ness (in)			
Number	Asphalt	Concrete	Aggregate Base	Sand Base	Total	Notes	
B-018	6	11	-	10	27		
B-019	6.5	8.5	-	5	20		
B-020	8.5	-	8.5	-	17		
B-021	9	-	5.5	-	14.5		
B-022	6.5	8.5	-	5	20		
B-023	6	8.75	-	5	19.75		
B-024	9	-	6	-	15		
B-025	9	-	6	-	15		
B-026	6.5	9	6	-	21.5		
B-027	6.5	10.5	-	-	17	Underlain by approx. 11 inches gravel fill with trace brick.	
B-028	6	10	-	6	22		
B-029	9	-	19	-	28		
B-030	7.5	9.5	-	3	20		
X-030-1	4.5	8.25	-	8	20.75	Eastbound Clark Road, west of SR44	
B-031	7	9	-	6	22		
B-032	5.5	9	-	5	19.5		
B-033	9	-	6	-	15		
B-034	9	-	6	-	15		
B-035	6.5	8.25	1	5	19.75		
B-036	7	8	-	5	20		
B-037	9	-	6	8	23		
B-038	4.5	9	-	5	18.5		
B-039	7	8	1	5	20		
B-040	6	7.5	1	11	24.5		
B-041	4	-	10	-	14		
B-042	8	-	7	-	15		
B-043	7	8	-	7	22		
B-044	4.5	9.25	-	6	19.75		
X-044-1	4.5	8.5	-	7	20	Eastbound Colburn Road, west of SR44	
B-045	12	-	-	4	16		
B-046	11	-	4	-	15		
B-047	5.25	8.5	-	6	19.75		
B-048	7	9	-	5	21		
B-049	7	8	-	5	20		



	Table 4.1. Surface Materials Summary							
Boring/Core		Pavemen	nt Component Thick	ness (in)	-			
Number	Asphalt	Concrete	Aggregate Base	Sand Base	Total	Notes		
B-050	-	-	-	-	-	Northbound SR44 potential widening. 2 inches topsoil, underlain by 7 inches crushed stone with slag and cinders, underlain by 6 inches sand.		
X-050-1	5.5	8	-	6	19.5	Northbound SR44, offset west of B-050.		
B-051	-	-	-	-	-	Southbound SR44 potential widening. 4 inches topsoil, underlain by 3 inches sand		
B-052	7	8	-	5	20			
B-053	5.5	8.5	-	5	19			
B-054	-	-	-	-	-	Northbound SR44 potential widening. 4 Inches aggregate at surface.		
B-055	-	-	-	-	-	Southbound SR44 potential widening. 4 Inches topsoil, underlain by 6 inches aggregate.		
B-056	3.25	9	-	4	16.25			
B-057	4	11	4	-	19			
B-058	7	8	-	-	15			
B-059	-	-	-	-	-	Boring was eliminated from scope of work.		
B-060	6.75	-	8.25	5	20	Westbound Girdled Road, west of SR44.		
B-061	3.75	8.5	-	5	17.25	Westbound Girdled Road, immediately east of SR44.		
B-062	-	-	-	-	-	Boring was eliminated from scope of work.		
B-063	6	-	7	-	13	Westbound Girdled Road, approx. 400 ft east of B-061.		
B-064	-	-	-	-	-	Boring was eliminated from scope of work.		
All Borings and	All Borings and Cores Data:							
Maximum	12	12	19	11	28			
Minimum	3.25	7.5	4	2	7			
Average	7	9	7	6	18			



	Table 4.1. Surface Materials Summary							
Boring/Core		Pavemer	nt Component Thick	ness (in)				
Number	Asphalt	Concrete	Aggregate Base	Sand Base	Total	Notes		
Intersection A	reas Boring	gs and Core	s Data:					
Maximum	Maximum 12 11 19 10 28							
Minimum	3.75	7.5	4	3	7			
Average	7	9	9	6	19			
Milling and Re	surfacing (	Only Areas I	Borings and Cores D	ata:				
Maximum	Maximum 10.5 12 10 11 25							
Minimum	3.25	7.5	4	2	13.75			
Average	7	9	6	6	18			

Existing fill materials were encountered in eight of the 61 borings, representing approximately 13 percent of the borings performed for this exploration. Fill materials were indicated where non-soil materials such as crushed stone or asphalt fragments were present in the soils. Granular fill materials classified as A-1-b or A-2-4 were encountered in four borings (B-027, B-034, B-042, and B-061). Cohesive fill materials classified as A-4a or A-6b were encountered in four borings (B-050, B-058, B-060, and B-063).

#### 4.2 <u>General Soil Conditions</u>

Based on the results of our field and laboratory tests, the subsoils encountered immediately underlying the surface materials predominantly consisted of cohesive soils. Cohesive subgrade soils were encountered in approximately 75 percent of the borings performed for this exploration. The cohesive soils were generally classified as A-4a or A-6a, although A-6b subgrade soils were encountered in one boring.

Native granular subgrade soils were encountered in 11 borings, representing approximately 18 percent of the test borings performed for this exploration. These granular soils were encountered in Borings B-002, B-003, B-004, B-008, B-012, B-017, B-024, B-025, B-033, B-041, and B-046. The native granular subgrade soils were classified as A-1-b and A-3a. It should be noted that encountered granular soils were more prevalent during this exploration than what was identified in the historic borings performed for the original construction of SR44 along this alignment. As such, some of the encountered granular soils may be fill materials placed to achieve design subgrades. However, non-soil materials were not observed in the recovered samples. Therefore, they have not been identified as fill.



Bedrock was encountered in nine test borings at depths ranging from 3 to 7 feet below existing grades. The borings in which bedrock was encountered and the depths at which bedrock was encountered are summarized in Table 2.1, in Section 2.1 of this report.

Descriptions of the stratigraphy encountered in the borings are presented on the Logs of Test Borings attached to this report. A summary of the subgrade soils encountered in the borings is also provided in tabular form in the Subgrade Analysis spreadsheet attached in Appendix A. The subgrade soils encountered immediately underlying the existing surface materials in each boring and the depths at which bedrock was encountered in the borings are summarized in Table 4.2 below. Borings performed in areas of intersections where widening is planned are highlighted in yellow.

	Table 4.2. Subgrade Soils Summary					
Boring/Core Number	Subgrade Soils Immediately Underlying Existing Pavement	Bedrock Depth Below Existing Grade (ft)				
B-001	A-4a	2.8				
B-002	A-3a	2.5				
B-003	A-3a	5.9				
B-004	A-3a	7.3				
B-005	A-6a	N.E.				
B-006	A-4a	N.E.				
B-007	A-4a	N.E.				
B-008	A-3a	N.E.				
B-009	A-6a	N.E.				
B-010	A-6a	N.E.				
B-011	A-6a	N.E.				
B-012	A-1-b	N.E.				
B-013	A-4a	N.E.				
B-014	A-6a	N.E.				
B-015	A-4a	N.E.				
B-016	A-6a	N.E.				
B-017	A-1-b	N.E.				
B-018	A-6a	N.E.				
B-019	A-6a	N.E.				
B-020	A-6a	N.E.				
B-021	A-6b	N.E.				
B-022	A-6a	N.E.				
B-023	A-4a	N.E.				
B-024	A-1-b	N.E.				



	Table 4.2. Subgrade Soils Summary					
Boring/Core Number	Subgrade Soils Immediately Underlying Existing Pavement	Bedrock Depth Below Existing Grade (ft)				
B-025	A-3a	6.5				
B-026	A-6a	N.E.				
B-027	A-2-4 (FILL)	N.E.				
B-028	A-6a	N.E.				
B-029	A-4a	N.E.				
B-030	A-4a	N.E.				
B-031	A-4a	N.E.				
B-032	A-6a	N.E.				
B-033	A-1-b	N.E.				
B-034	A-1-b (FILL)	5.8				
B-035	A-4a	N.E.				
B-036	A-6a	N.E.				
B-037	A-6a	N.E.				
B-038	A-6a	N.E.				
B-039	A-6a	N.E.				
B-040	A-6a	N.E.				
B-041	A-3a	N.E.				
B-042	A-1-b (FILL)	N.E.				
B-043	A-4a	N.E.				
B-044	A-4a	N.E.				
B-045	A-6a	7.0				
B-046	A-3a	N.E.				
B-047	A-6a	N.E.				
B-048	A-6a	N.E.				
B-049	A-4a	N.E.				
B-050	A-6b (FILL)	N.E.				
B-051	A-4a	4.1				
B-052	A-6a	4.5				
B-053	A-6a	N.E.				
B-054	A-6a	N.E.				
B-055	A-4a	N.E.				
B-056	A-4a	N.E.				
B-057	A-4a	N.E.				
B-058	A-4a (FILL)	N.E.				
B-060	A-4a (FILL)	N.E.				
B-061	A-2-4 (FILL)	N.E.				
B-063	A-4a (FILL)	4.0				



#### 4.3 **Groundwater Conditions**

Groundwater was initially encountered during drilling operations in only eight of the 61 test borings performed for this exploration, at depths ranging from approximately  $1\frac{1}{2}$  to  $6\frac{1}{2}$  feet below existing grades. Groundwater was observed upon completion of drilling four of these borings at depths ranging from approximately  $1\frac{1}{2}$  to  $6\frac{1}{2}$  feet. The groundwater conditions encountered in the borings are summarized in the following table.

	Table 4.3. Encountered Groundwater Conditions						
Boring Number	Depth of Groundwater Initially Encountered During Drilling (feet)	Depth of Groundwater Observed At Completion of Drilling (feet)					
B-003	3.5	3.1					
B-009	1.5	N.E.					
B-013	1.3	N.E.					
B-017	6.5	6.5					
B-033	5.0	4.8					
B-034	4.8	N.E.					
B-042	5.5	N.E.					
B-045	3.3	1.5					

N.E. = Not Encountered

It should be noted that each of the borings was drilled and backfilled within the same day. As such, stabilized water levels may not have occurred over this limited time period. Instrumentation was not installed to observe long-term groundwater levels.

Based on the limited data available, such as the soil characteristics and the groundwater conditions encountered in the borings, it is our opinion that the "normal" groundwater level is anticipated to generally be encountered at depths on the order of 5 feet or greater below existing grades. This exploration did not include research of possible hydrological influences at the project site. It should be noted that groundwater elevations can fluctuate with seasonal and climatic influences. In particular, "perched" water may be encountered in fill materials or granular soils that are underlain by relatively impermeable native cohesive soils, or at the transition from soil to bedrock (particularly in weathered rock zones). Therefore, the groundwater conditions at the site may vary at different times of the year from those encountered during this investigation.

#### 4.4 Remedial Measures

Based on the ODOT "Subgrade Analysis" worksheet (V14.6, 02/11/2022), 15 of the 25 borings in intersection areas of planned widening contained subgrade soils within the upper profile which indicated subgrade modification is likely to be required. Three (3) of these borings indicated planned subgrade modification consisting of reworking granular soils in-place. Based on the GB-1 (Now ODOT Geotechnical Design Manual Section 600) analysis results, subgrade modification may consist of global chemical stabilization using cement to a depth of 14 inches, or over-excavation and replacement with new granular engineered fill. With widening planned for widths generally 12 feet or less, it is not expected that global chemical stabilization equipment could readily access and treat the subgrades. As such, it is planned that subgrade modification for this project include over-excavation and replacement with new granular engineered fill.



#### 5.0 ANALYSES AND RECOMMENDATIONS

The following analysis and recommendations are based on our understanding of the proposed construction and on the data obtained during our field exploration. If the project alignment or subgrade depth should change significantly, a review of these recommendations should be made by TTL.

#### 5.1 GB-1 "Plan Subgrades" Evaluation

An evaluation of the subgrade soils was completed in general accordance with ODOT Geotechnical Bulletin GB-1 "Plan Subgrades" (Now Geotechnical Design Manual Section 600). As part of this evaluation, the ODOT "Subgrade Analysis" worksheet (V14.6, 02/11/2022) was completed for the project and is attached to this report in Appendix A.

Final roadway grades will approximate existing roadway grades and consist of asphalt pavements. New pavement widening areas are planned to include 9½ inches of asphalt underlain by 6 inches of aggregate base, for a total cross-section of 15½ inches (approximately 1.3 feet). As such, the "cut" depth in the Subgrade Analysis spreadsheet is generally 1.3 feet. In the area of Boring B-047, minor grading is planned which resulted in a "cut" depth to the planned subgrade elevation of 1.1 feet.

Based on GB-1/GDM Section 600, soils classified as ODOT A-4b, A-2-5, A-5, A-7-5, A-8a, A-8b, or rock have been designated as being problematic with respect to pavement subgrade support. Of these, only rock was encountered during this exploration at subgrade elevations. Although rock was encountered in 10 test borings, it was present within 24 inches of the subgrade elevation only in Borings B-001 and B-002. These two borings were <u>not</u> performed in intersection areas of widening.

The subgrade materials encountered in the borings included predominantly cohesive soils consisting generally of ODOT A-4a and A-6a soils, with some zones of A-6b soils. Approximately two-thirds of the borings included subgrade soils immediately underlying the existing surface materials consisting of granular soils. The granular soils generally consisted of ODOT A-3, A-3a, A-1-b, or A-2-4.

Based on GB-1/GDM Section 600 criteria, subgrade soils with moisture contents greater than 3 percent above optimum likely indicate the presence of unstable subgrade that may require some form of subgrade modification. Moisture contents for approximately 50 percent of the tested subgrade soil samples were greater than 3 percent above the optimum as determined



using GB-1 criteria. It should be noted that approximately two-thirds of the evaluated samples with moisture contents greater than 3 percent above optimum had moisture contents equal to or greater than 5 percent above optimum. Thus, where moisture contents were wet of optimum, they were typically significantly wet of optimum. Scarification and aeration methods may be considered for the granular soils, provided the construction schedule and weather conditions are conducive to such modification. However, scarification and aeration methods may not be feasible to achieve satisfactory proof rolling and stabilization of the cohesive subgrades.

The type and thickness of subgrade modification is determined by GB-1 criteria based on the average, low SPT  $N_{60}$ -value ( $N_{60L}$ ) of the subgrade soils in a particular portion of the project area, hand penetrometer values, soil type, and moisture content. Based on these criteria, 15 of the 25 borings in intersection areas where widening is planned (60 percent of the borings) contained subgrade soils within the upper profile which indicated subgrade modification is likely to be required. Of the 15 borings indicating likely need of modification, three (3) of these borings contained granular soils that are designated to be re-compacted in-place.

Based on the GB-1 analysis results, subgrade modification may consider global chemical stabilization using cement to a depth of 14 inches, or over-excavation and replacement with new granular engineered fill. With more than 30 percent of the project indicating likely need for modification, consideration was given to the global chemical stabilization option. The sulfate content test results for subgrade soil samples ranged from 270 parts per million (ppm) to less than 100 ppm, which would not preclude use of chemical stabilization. However, with widening planned for widths generally 12 feet or less, it is not expected that global chemical stabilization equipment could readily access and treat the subgrades. As such, it is planned that subgrade modification for this project include over-excavation and replacement with new granular engineered fill.

A summary of the depths of undercut indicated by GB-1/ODOT GDM Section 600 analyses is presented in the following tables for SR44 and Girdled Road, respectively.



Table 5.1.A GB-1 Recommended Depth of Undercut and Replacement with Granular Engineered Fill SR44					
Boring Number  GB-1 Recommended Depth of Undercut and Replacement with Granular Engineered Fill (inches)		Recommended Subgrade Modification Extents	Approximate Project Segment Length (feet)		
B-014 & B-015	12	LT: 81+21.77 to 85+48.58 RT: 80+58.97 to 84+93.51	427 435		
B-030	18	LT: 139+00 to 140+28.62 RT: 139+00 to 140+07.11	129 107		
B-031	12	LT: 143+87.76 to 147+51.86 RT: 142+90.84 to 147+44.74	364 454		
B-048 & B-049	24	LT: 209+50 to 216+59.95 RT: 211+08.83 to 217+27.17	710 618		
B-050	15	LT: 217+38.24 to 221+50 RT: 218+30.02 to 221+50	412 320		
B-051	12	LT: 221+50 to 224+40.81 RT: 221+50 to 225+50	291 400		
B-053	18	RT: 229+00 to 229+72.00	72		

Table 5.1.B GB-1 Recommended Depth of Undercut and Replacement with Granular Engineered Fill Girdled Road			
Boring Number	GB-1 Recommended Depth of Undercut and Replacement with Granular Engineered Fill (inches)	Recommended Subgrade Modification Extents	Approximate Project Segment Length (feet)
Nearby B-050 <sup>1</sup>	15	LT: 104+48.07 to 106+87.25	239
Nearby B-049 <sup>2</sup>	24	RT: 101+90.36 to 104+55.65	265

<sup>&</sup>lt;sup>1</sup>Nearest boring along Girdled Road is B-061, which indicates 12 inches of planned undercut. Since B-050 was performed just north of this widening area along SR44, and indicated slightly more conservative undercut depth of 15 inches, the recommendation is provided based on B-050.

It should be noted that, in the above tables, transitions were generally based on the extent of planned widening. As such, stationing is indicated to the nearest hundredth of foot. Where transitions are planned between borings indicating areas of recommended treatment and borings indicating no treatment, or varying undercut depth, the transition was indicated approximately half way between borings – conservatively rounded to the nearest whole 50 feet that would provide more subgrade treatment.

<sup>&</sup>lt;sup>2</sup>Nearest boring along Girdled Road is B-060, which was performed along left edge of roadway and indicates no planned undercut. Since B-049 was performed just south of this widening area along the west side of SR44, it is considered more representative of the material that may be present along the right edge of Girdled Road in this area of widening. Therefore, B-049 was used for the recommended undercut condition.

Where undercut and replacement is utilized, all fill should consist of ODOT Item 304 Aggregate Base or Item 703.16C, Granular Material Type B or Type C. It is recommended that geotextile fabric (referenced in ODOT Item 204, and specified as ODOT Item 712.09, Type D) be utilized on the subgrade at the bottom of the undercut zone. If particularly unstable subgrades are encountered during construction, or undercuts are on the order of 18 inches or greater, a geogrid could be used to reduce the total undercut and replacement of the unsuitable soils by 6 inches.

It should be noted that GB-1 analyses are used as a pre-construction tool to plan subgrade modification alternatives. **Actual subgrade modification will depend on field observations of proof-rolling conditions at the time of construction.** Changes in soil moisture content could create more or less favorable subgrade conditions that may result in adjustments to subgrade modification or soil stabilization requirements at the time of construction.

#### 5.2 Flexible (Asphalt) Pavement Design

The ODOT "Subgrade Analysis" worksheet for the entire project site resulted in a CBR value of 7 percent. It should be noted that the CBR determination by the Subgrade Analysis spreadsheet is based on an average Group Index (GI) of all the evaluated samples. It should be noted that group indices for the tested samples ranging from 9 to 16, which would correlate with lower CBR values than that determined based on the average GI value of 7, were generally encountered in borings that were indicated by the Subgrade Analysis spreadsheet for subgrade modification, or were present at depths of at least 1½ feet below the anticipated top of subgrade elevation. As such, it does not appear to be unconservative to use the Subgrade Analysis worksheet design CBR value of 7 percent based on the average design value calculations.

It should also be noted that the design CBR value is based on subgrades compacted to at least 100 percent of the maximum dry density as determined by ASTM D 698 (Standard Proctor) or verified as stable through proof-rolling in accordance with Section 5.3 of this report.

All pavement design and paving operations should conform to ODOT specifications. The pavement and subgrade preparation procedures outlined in this report should result in a reasonably workable and satisfactory pavement. It should be recognized, however, that all pavements need repairs or overlays over time as a result of progressive yielding under repeated loading for a prolonged period.



It is recommended that proof rolling, placement of aggregate base, and placement of asphalt be performed within as short a time period as possible. Exposure of the aggregate base to rain, snow, or freezing conditions may lead to deterioration of the subgrade and/or base materials due to excessive moisture conditions and to difficulties in achieving the required compaction.

#### 5.3 Site and Subgrade Preparation

Site and subgrade preparation activities should conform to ODOT Construction and Materials Specifications (CMS) Item 204 specifications. Site preparation activities should include the removal of vegetation, topsoil, root mats, pavements, and other deleterious non-soil materials from all proposed roadway areas. The actual amount of required stripping should be determined in the field by a geotechnical engineer or qualified representative.

Upon completion of the clearing and undercutting activities, all areas that are to receive fill, or that have been excavated to proposed final subgrade elevation, should be inspected by a geotechnical engineer. Pavement subgrades should be proof rolled in accordance with ODOT CMS 204.06.

Any unsuitable materials observed during the inspection and proof-rolling operations should be undercut and replaced with compacted fill, or stabilized in place utilizing conventional remedial measures such as discing, aeration, and recompaction. As stated previously, based on the conditions encountered during our exploration, where subgrade soil moisture contents were wet of optimum, they were generally significantly wet of optimum. The encountered granular subgrade soils should be generally conducive for subgrade modification consisting of scarification, aeration, and in-place re-compaction, provided weather conditions and construction schedule will allow for these activities. However, scarification and aeration methods may not be feasible to achieve satisfactory proof rolling and stabilization of the cohesive subgrades.

The GB-1 (Now ODOT Geotechnical Design Manual Section 600) analysis indicates options for "planned" subgrade modification consisting of global chemical stabilization using cement to a depth of 14 inches, or over-excavation of unsuitable subgrade soils and replacement with new granular engineered fill. With widening planned for widths generally 12 feet or less, it is not expected that global chemical stabilization equipment could readily access and treat the subgrades. As such, it is planned that subgrade modification for this project include over-excavation and replacement with new granular engineered fill.



#### 5.4 Groundwater Control

Encountered groundwater conditions were previously discussed in Section 4.3. Based on the limited data available, such as the soil characteristics and the groundwater conditions encountered in the borings, it is our opinion that the "normal" groundwater level is anticipated to generally be encountered at depths of 5 feet or greater below existing grades. It should be noted that "perched" water may be encountered in the fill materials or granular soils that are underlain by relatively impermeable native cohesive soils, or at the transition from soil to bedrock (particularly in weathered rock zones).

If construction does not occur during a particularly wet period, adequate control of groundwater seepage into excavations extending only a few feet below the "normal" groundwater level should be achievable by minor dewatering systems, such as pumping from prepared sumps. Even at depths slightly below the "normal" groundwater level, control of groundwater using sumps should be feasible due to the predominantly cohesive nature of the encountered soils and their associated low permeability, but will require due diligence by the contractor to maintain a stable subgrade condition at the bottom of the excavation. If saturated granular soils are encountered, they may require installation of well points in addition to pumping from prepared sumps.

#### 5.5 Excavations and Slopes

The sides of temporary excavations for construction should be adequately sloped to provide stable sides and safe working conditions. Otherwise, the excavation must be properly braced against lateral movements. In any case, applicable Occupational Safety and Health Administration (OSHA) safety standards must be followed.

Based on the test borings, the soils likely to be encountered in shallow excavations may include:

- OSHA Type A soils (cohesive soils with unconfined compressive strengths of 3,000 pounds per square foot (psf) or greater),
- OSHA Type B soils (cohesive soils with unconfined compressive strengths greater than 1,000 psf but less than or equal to 3,000 psf, as well as dry rock that is not stable), and
- OSHA Type C soils (cohesive soils with unconfined compressive strengths less than 1,000 psf, existing fill materials, and granular soils).



Temporary excavations in Type A, B, and C soils should be constructed no steeper than ¾ horizontal to 1 vertical (¾H:1V), 1H:1V, and 1½H:1V, respectively. For situations where a higher strength soil overlies a lower strength soil, and the excavation extends into the lower strength soil, the slope of the entire excavation is governed by that required for the lower strength soil. In all cases, flatter slopes may be required if lower strength soils or adverse seepage conditions are encountered during construction.

For permanent excavations and slopes, we recommend that grades generally be no steeper than 3H:1V. It should be noted that ODOT routinely uses 2H:1V slopes for roadway embankments. While these steeper slopes may be used, it is our experience that the embankment faces on these slopes are more prone to erosion and sloughing.

#### **5.6** Fill

Material for engineered fill or backfill required to achieve design grades should meet ODOT Item 203 "Embankment Fill" placement and compaction requirements. In general, suitable fills may consist of any non-organic soils having a maximum dry density as determined by the Standard Proctor (ASTM D 698) of 90 pounds per cubic foot (pcf) or greater. Additionally, fill utilized to achieve design grades should consist of materials similar to, or better than, the onsite soils. Otherwise, a reduced CBR value may be required for pavement design.

On-site soils may be used as engineered fill materials provided that they are free of organic matter, debris, excessive moisture, and rock or stone fragments larger than 3 inches in diameter. Depending on seasonal conditions, the on-site soils may be wet of optimum and may require scarification and aeration to achieve satisfactory compaction. However, if the construction schedule does not allow for scarification and aeration activities, it may be more practical or economical to utilize imported granular fill.

Fill should be placed in uniform layers not more than 8 inches thick (loose measure) and adequately keyed into stripped and scarified soils. Based on the planned widening, some areas of new embankment fill will require special benching per ODOT GB-2 (Now Geotechnical Design Manual Section 800). The project plans include recommendations regarding special benching. All fill placed within pavement areas should be compacted to a dry density consistent with the requirements of ODOT Item 203, based on the maximum dry density as determined by ASTM D 698.



The on-site soils consist of both cohesive and granular soils. For the cohesive soils, a sheepsfoot roller should provide the most effective soil compaction. For granular soils, as well as dense-graded aggregate pavement base materials, a vibratory smooth-drum roller would be required to provide effective compaction.

Scarified subgrade soils and all fill material should be within 3 percent of the optimum moisture content to facilitate compaction. Furthermore, fill material should not be frozen or placed on a frozen base. It is recommended that all earthwork and site preparation activities be conducted under adequate specifications and properly monitored in the field by a qualified geotechnical testing firm.



#### 6.0 QUALIFICATION OF RECOMMENDATIONS

Our evaluation of the pavement design and construction conditions has been based on the data obtained during our field exploration, as well as the criteria in ODOT Geotechnical Bulletin GB-1 "Plan Subgrades" (Now Geotechnical Design Manual Section 600). The general subsurface conditions were based on interpretation of the pavement and subsurface data at specific core/boring locations. Regardless of the thoroughness of a subsurface exploration, there is the possibility that conditions between core/borings will differ from those at the boring locations, that conditions at the time of construction are not as anticipated by the designers, or that the construction process has altered the soil conditions. This potential is increased for previously developed sites. Therefore, experienced geotechnical engineers should observe earthwork and pavement construction to confirm that the conditions anticipated in design are noted. Otherwise, TTL assumes no responsibility for construction compliance with the design concepts, specifications, or recommendations.

The design recommendations in this report have been developed on the basis of the previously described project characteristics and subsurface conditions. If project criteria or locations change, TTL should be permitted to determine whether the recommendations must be modified. The findings of such a review will be presented in a supplemental report.

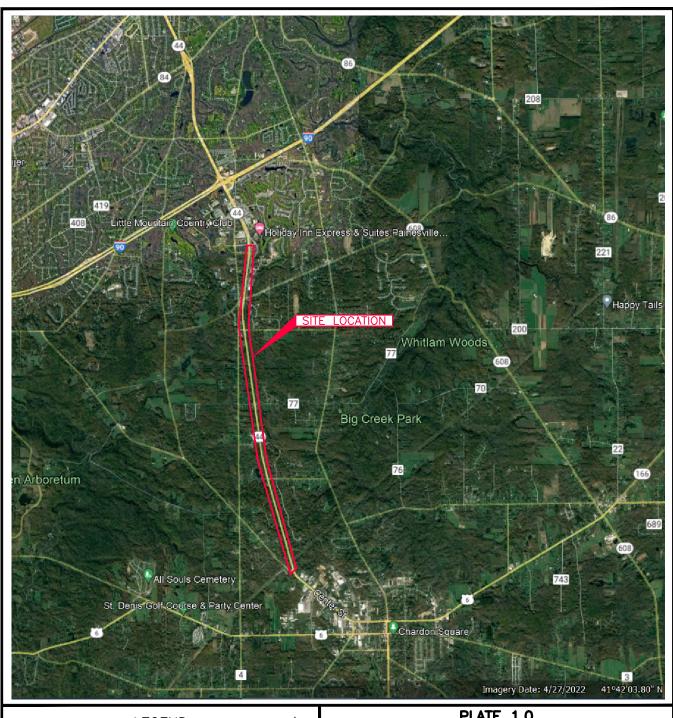
The nature and extent of variations between the borings may not become evident until the course of construction. If such variations are encountered, it will be necessary to reevaluate the recommendations of this report after on-site observations of the conditions.

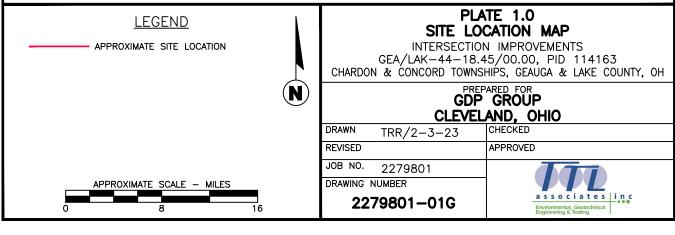
Our professional services have been performed, our findings derived, and our recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties either expressed or implied. TTL is not responsible for the conclusions, opinions, or recommendations of others based on this data.

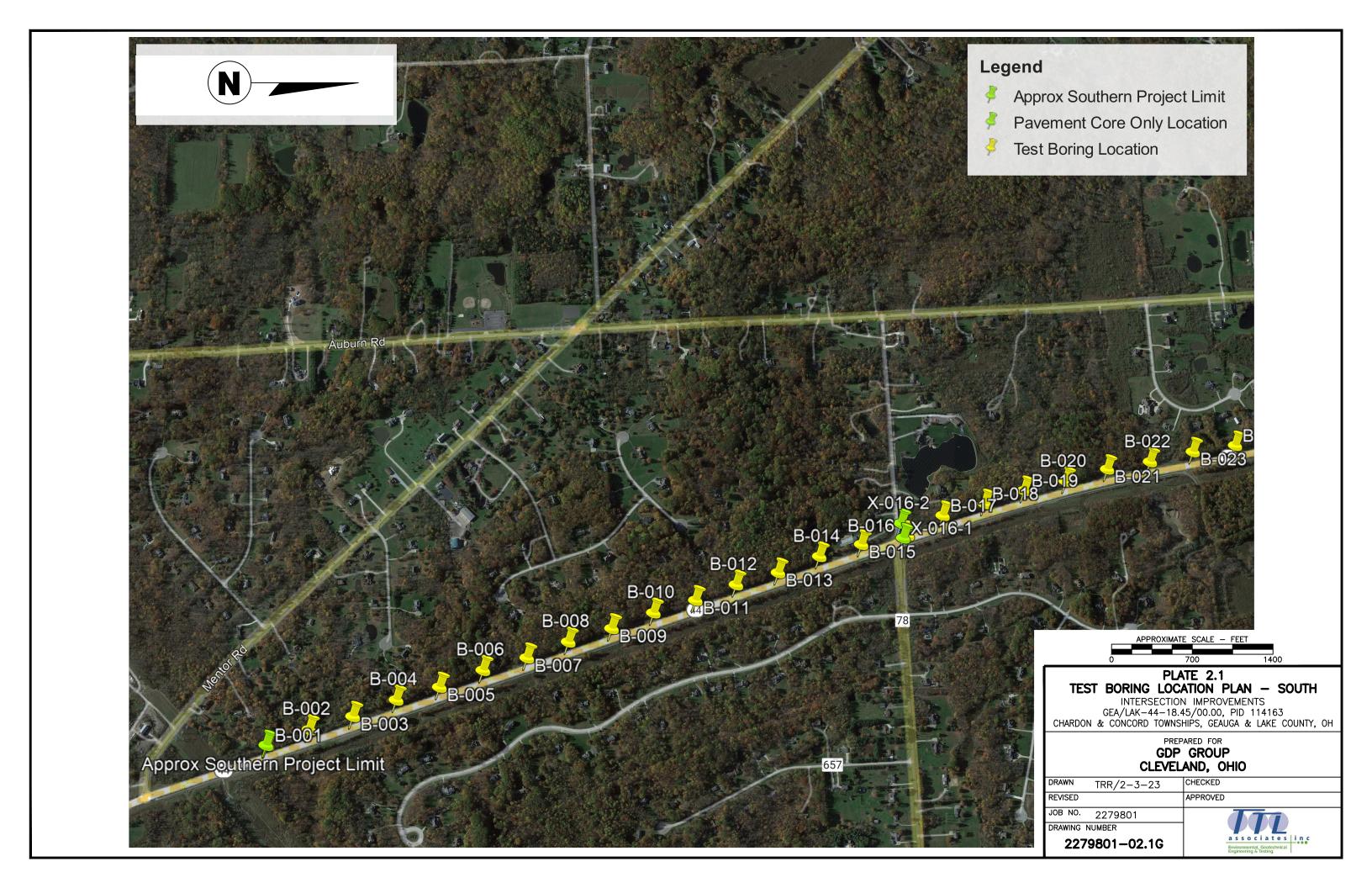


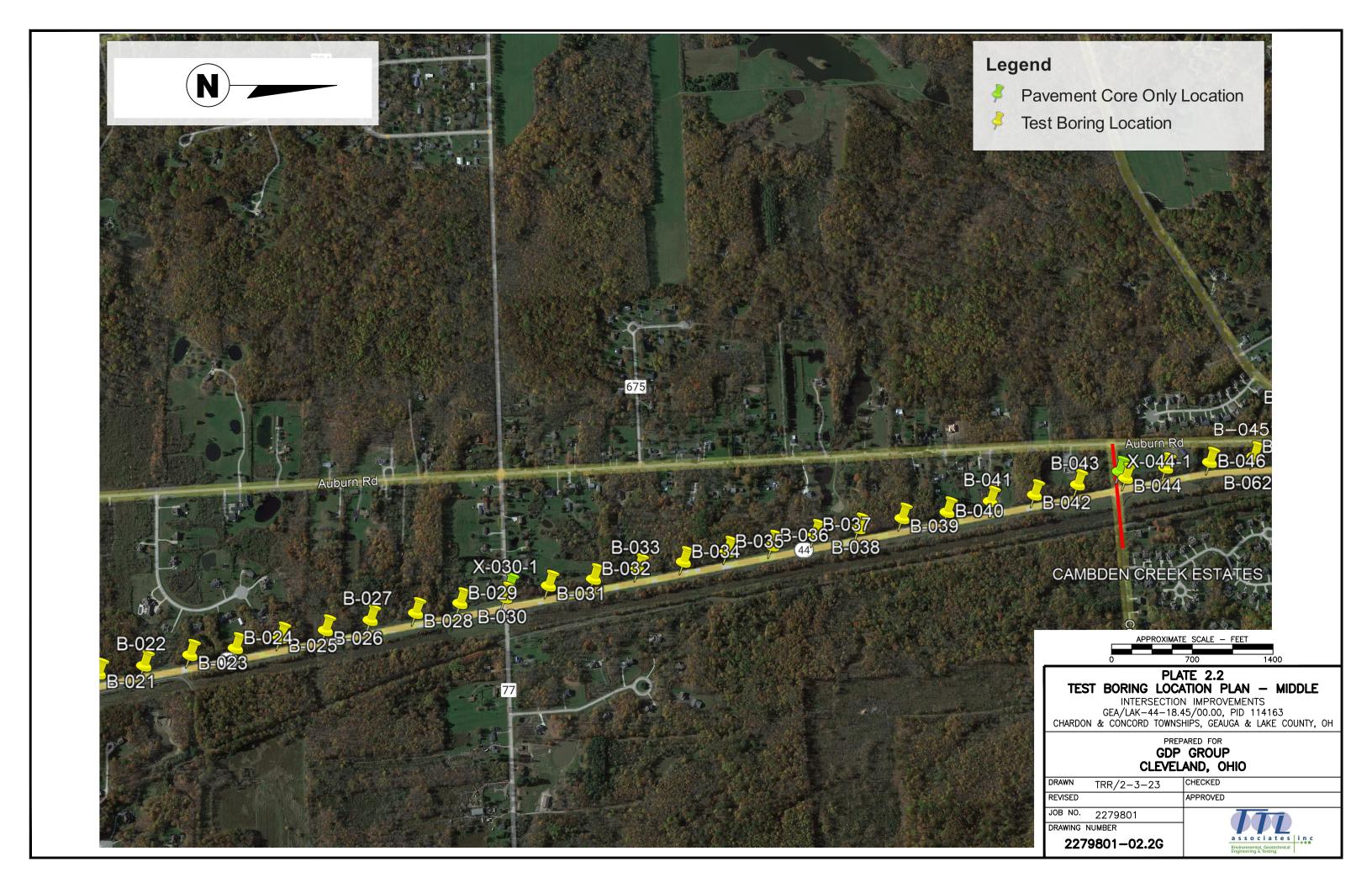
#### **PLATES**

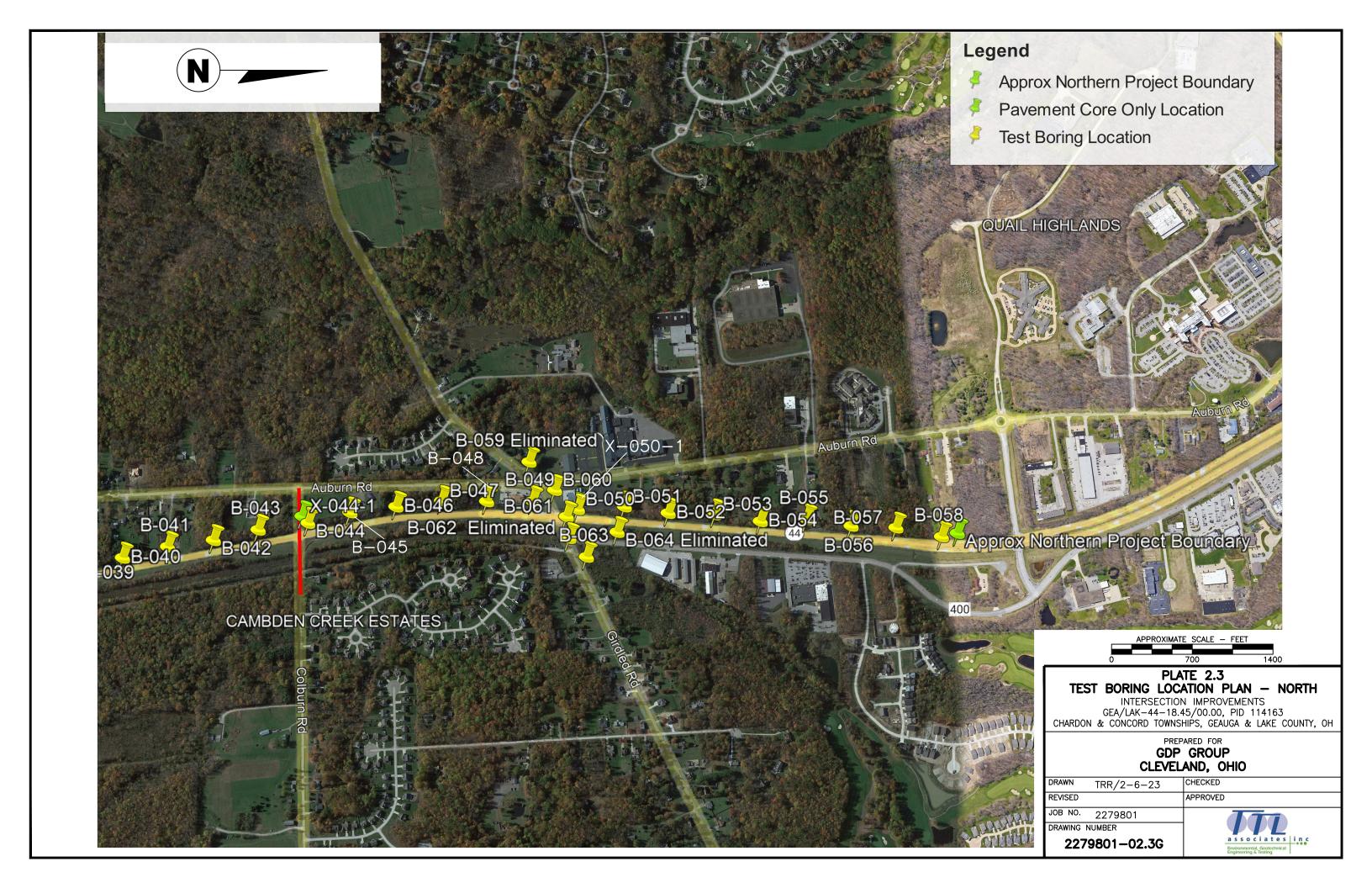


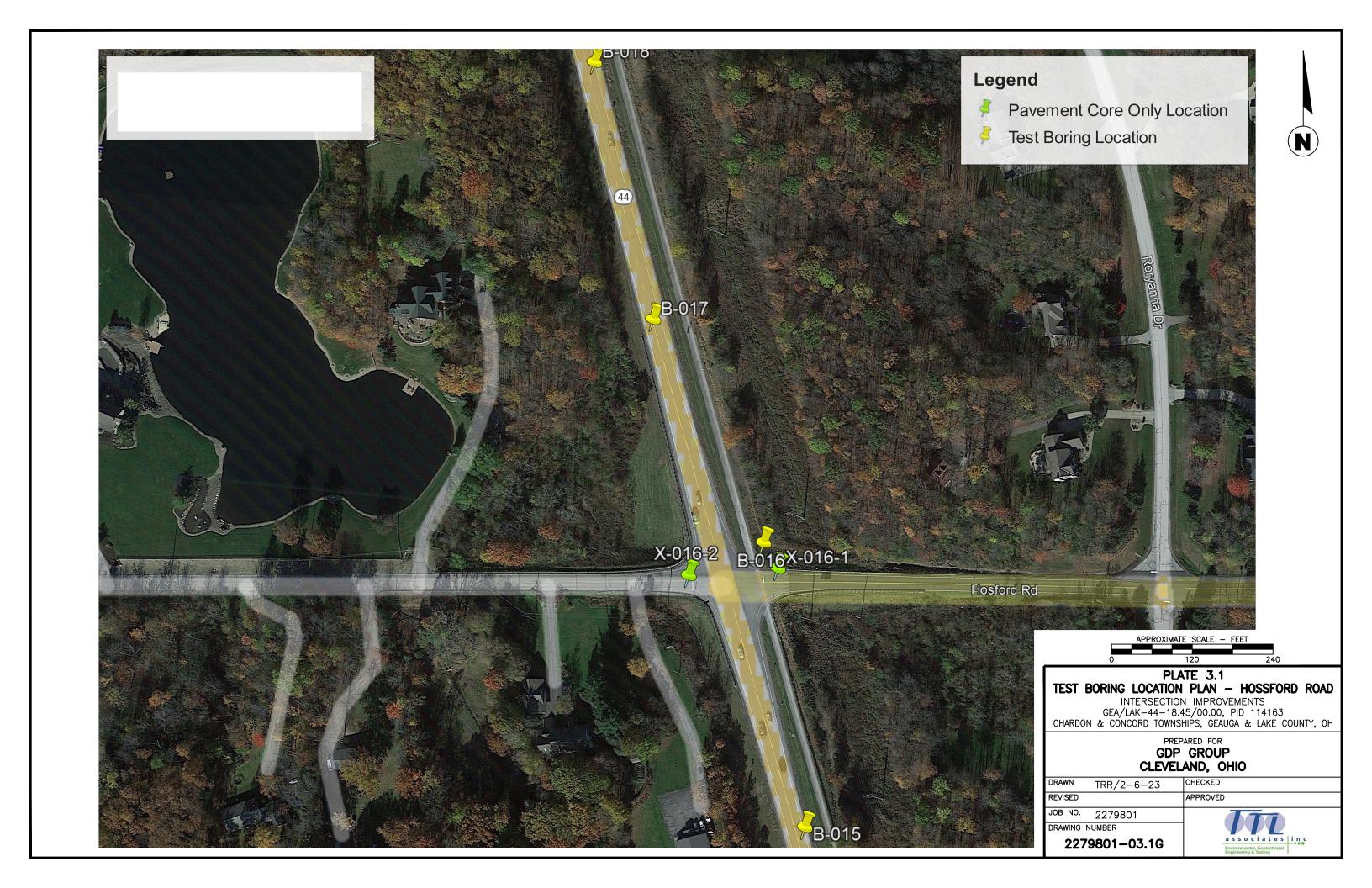




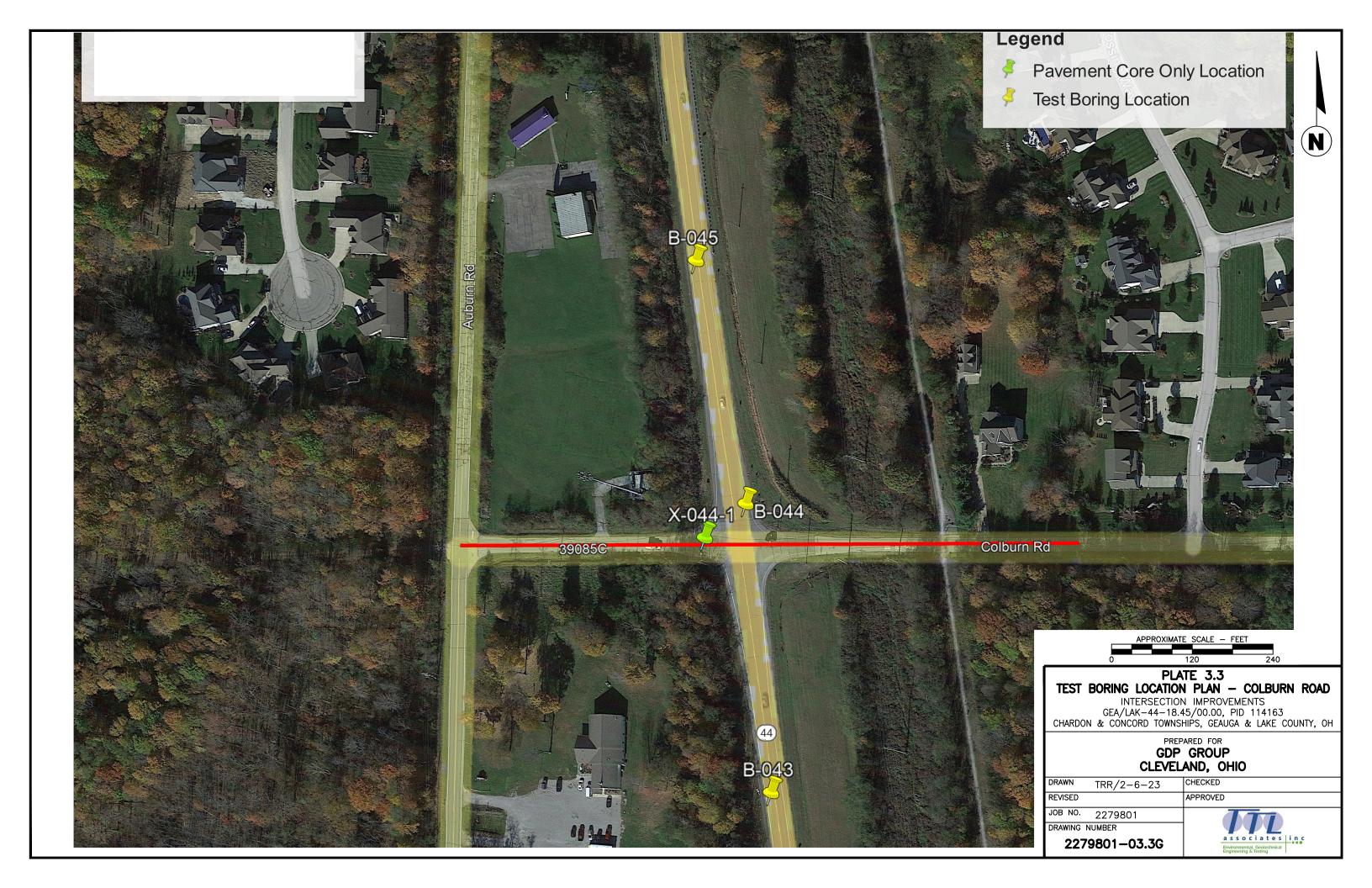


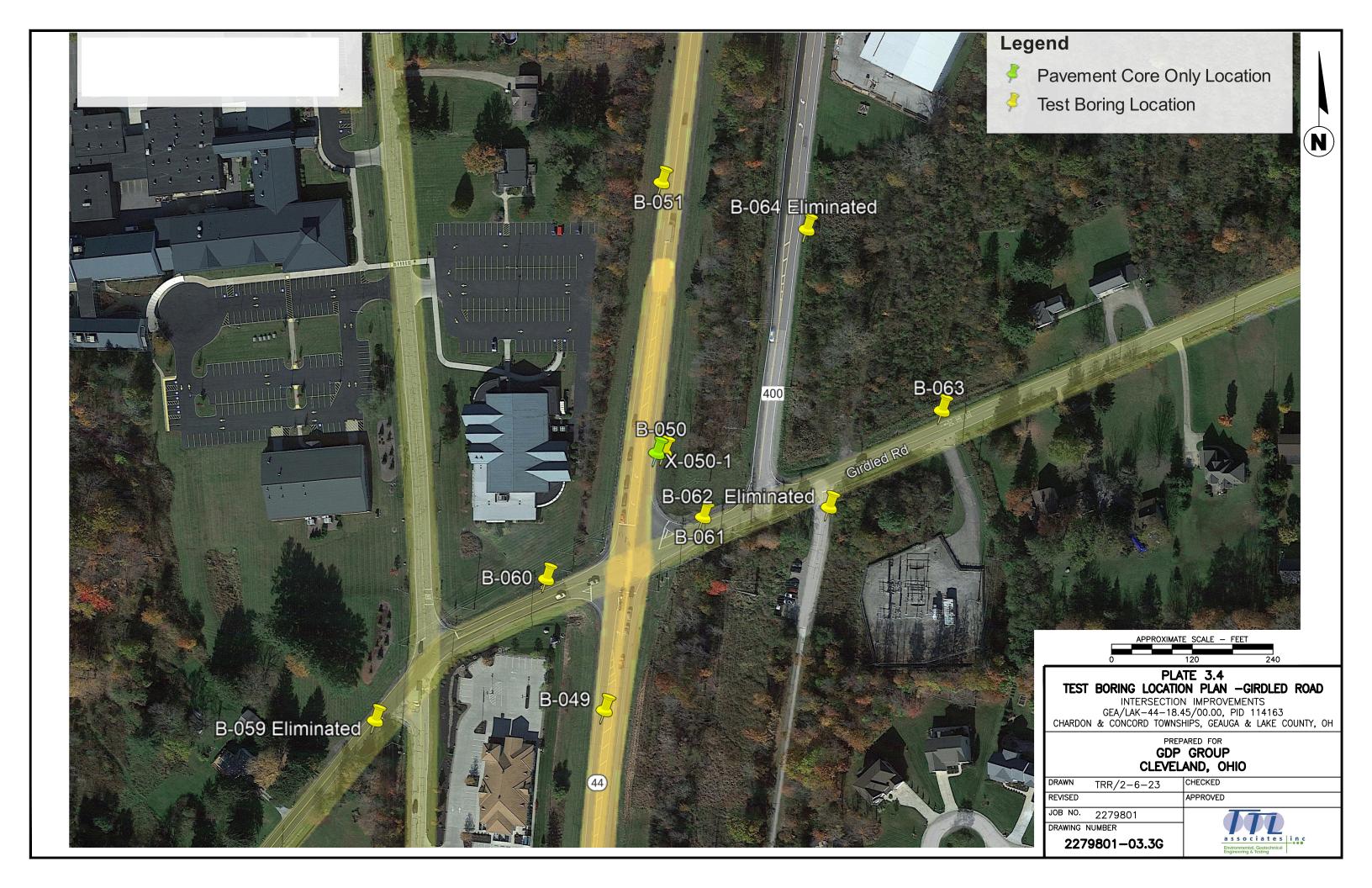












# FIGURES (INTERSECTIONS BORINGS/CORES)



DRILLING N SAMPLING SCRIPTION DIES  LT SOME SAND, TRACE			3.5" SP	Т	SPT/ RQD		RGY R	ON DATE: ATIO (%): SAMPLE ID	HP (tsf)	66 C	SRAD	COOR ATION FS S	D: (%)	7104 ATT	29.69 ERBI	910 N ERG		ODOT CLASS (GI)	SO4 ppm
LT SOME SAND, TRACE	· F. G.	1178.5	DEPTI	- 1 - - 1 -	SPT/ RQD		REC	SAMPLE ID	HP (tsf)								WC		
LT SOME SAND, TRACE		1178.0	DEFI	- 1 - - 1 -	RQD	1460	(%)			GR	CS	FS S	I CL	LL	PL	PI	WC	CLASS (GI)	ppm
SOME SAND, TRACE	: F.S.:	1177.3		<u> </u>	-			00.44											
SOME SAND, TRACE	F.S.	1177.3		<u> </u>				00.41											
SOME SAND, TRACE	ir ş			<u> </u>				00.44											
SOME SAND, TRACE	F S.			<u> </u>				00.44											
SOME SAND, TRACE	FS							00.44											
SOME SAND, TRACE	F.S.	1176.9						00.44											
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		1		F 1	5														
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		1 1		<b>├</b> 4 ┤	· '														
		1 1																	
				F 1										1					
		1173.5						SS-3A	-	-	-	-   -	-	-	-	-	-	A-6a (V)	-
, COARSE AND FINE		1170.0		<u></u> 5	7									1					
Y, MOIST					9	22	100												
					I ''			SS-3B	-	-	-	-   -	-	-	-	-	14	A-3a (V)	-
		1172.5																	
ILT, LITTLE CLAY,				F 6 7															
AMP																			
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				,			100	55-4	-	-	-	-   -	-	-	-	-	13	A-4a (V)	-
				$\Gamma'$	·-														
		1171.0	FOR																
IL7	T, LITTLE CLAY,	r, LITTLE CLAY,	r, LITTLE CLAY,	r, LITTLE CLAY,	7, LITTLE CLAY, P	7, LITTLE CLAY, P	7, LITTLE CLAY, P	1172.5 F, LITTLE CLAY, P  1172.5  - 6  - 8 10 - 7 - 12  24 100	T, LITTLE CLAY, P  1172.5  - 6  - 8  10  24  100  SS-3B	T, LITTLE CLAY, P  1172.5  - 6  - 8  10  24  100  SS-3B  - 10  1171.0	7, LITTLE CLAY, P	1172.5   SS-3B   -   -   -	T, LITTLE CLAY, P  8 10 1171.0	T, LITTLE CLAY, P  8 10 1171.0	T, LITTLE CLAY, P  8 10 1171.0	T, LITTLE CLAY, P  1172.5  8 10 24 100 SS-3B	T, LITTLE CLAY,  P  8 10 24 100 SS-4	T, LITTLE CLAY, P  8 10 24 100 SS-4 13	T, LITTLE CLAY,  P  1172.5  6  8 10 12 24 100 SS-4

EXPLORATION ID DRILLING FIRM / OPERATOR: PROJECT: GEA/LAK-44-18.45/00.00 TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 82+95, 9' RT. B-015-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1166.9 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/14/22 END: COORD: 12/14/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 710809.3970 N, 2314596.3830 E ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1166.9 ASPHALT - 9.5 INCHES 1166.1 AGGREGATE BASE - 7.5 INCHES 1 · W Span 1165.5 LOOSE, BROWN, SANDY SILT, TRACE CLAY, TRACE GRAVEL, DAMP 2 NP NP SS-1A 6 34 18 34 NP A-4a (1) 10 100 1164.1 SS-1B A-4a (V) MEDIUM STIFF, BROWN, SANDY SILT, SOME CLAY, 3 TRACE GRAVEL, TRACE SHALE FRAGMENTS, DAMP SS-2A 4 15 10 40 31 24 16 8 14 A-4a (7) 1163.6 LOOSE, BROWN, COARSE AND FINE SAND, SOME CLAY, LITTLE SILT, MOIST 8 100 SS-2B 12 A-6a (V) <100 444p SS-3A A-3a (V) 1161.9 5 N SED MEDIUM DENSE, BROWN, FINE SAND, TRACE SILT SS-3B A-3 (V) 1161.6 21 100 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, SOME GRAVEL. DAMP SS-3C 9 A-6a (V) 1160.9 400 6 HARD, GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP WSDD 16 20 45 100 SS-4 4.50 12 A-6a (V) ا ح آلا 21 1 2 1159.4 CONTRACT. NOTES: NONE

PROJECT:	ROADWAY	DRILLING FIRM / C	LOGGER: _	TTL / KKC	HAMMER: CME AUTOMATIC							STATION / OFFSET: 87+11, 59' RT. ALIGNMENT: SR44 ELEVATION:1152.8 (NAVD88)EOB:							EXPLORATION IE B-016-0-22 10.0 ft. PAGE		
PID: <u>114163</u> START: 12/		DRILLING METHO SAMPLING METHO		3.5" SSA SPT				ON DATE: ATIO (%):		/15/2 66	1	COO						<u>10.0 π</u> 1525.2730 E		1 OF 1	
017 ((1: <u>12/</u>	MATERIAL DESCRIPTION		ELEV.		SPT/			SAMPLE	_		GRAF	OITAC				BERG		ODOT	SO4	BACK	
	AND NOTES	<i>5</i> 11	1152.8	DEPTHS	RQD	N <sub>60</sub>	(%)	ID	(tsf)		_				L PI		wc	CLASS (GI)		FILL	
TOPSOIL - 3			1152.5																	STAP Pg L gg	
HARD, GRAY GRAVEL, DA	Y, <b>SILT AND CLAY</b> , SOME SA MP	AND, TRACE	1102.0	- 1 7																4 × × × × × × × × × × × × × × × × × × ×	
				_ 2 -	3 5 8	14	100	SS-1	4.50	8	8	14	23	47 2	6 1	5 11	11	A-6a (7)	260	2 L X 2 L X X X X X X X X X X X X X X X	
			1149.3	_ 3 _	-																
STIFF, BROV TRACE GRAV	WN, <b>SILT AND CLAY</b> , LITTLE VEL, TRACE SHALE FRAGM			<u> </u>	1 2 3	6	100	SS-2	1.25	3	9	9	22	57 2	9 1	7 12	18	A-6a (9)	-		
TRACE GRAV				_ 5 _	-																
			1146.1	— 6 <u>—</u>	3 4	11	100	SS-3A	-	-	-	-	-			-	-	A-6a (V)	-	1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	
VERY STIFF, TRACE GRAV	, GRAY, <b>SILTY CLAY</b> , LITTLE VEL, DAMP	SAND,		<del>-</del> 7 -	6		100	SS-3B	2.50	-	-	-	-	-   .		-	16	A-6b (V)	-	7 L	
			1144.3	_ 8 -																1 × × × × × × × × × × × × × × × × × × ×	
	, GRAY/BROWN, <b>SILT AND (</b> E GRAVEL, DAMP	CLAY, LITTLE		_ 9 - -	8 13 11	26	100	SS-4	-	-	-	-	-		.   -	-	14	A-6a (V)	-		
NOTES: NO	<u>ONE</u> ENT METHODS, MATERIALS	OHANTITIES: AUG	1142.8	EOB 10	0.5.040	DEN	TONUT			1	1						1	1	1		

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 90+74, 9' LT. B-017-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1143.5 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/14/22 END: COORD: 12/14/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 711551.2100 N, 2314356.5910 E REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1143.5 ASPHALT - 7 INCHES 1142.9 MEDIUM DENSE. BROWN. GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, - 1 -DAMP NP SS-1A 49 26 17 2 NP NP 2 6 A-1-b (0) 17 100 SS-1B <100 A-1-b (V) 3 1140.3 SS-2A A-1-b (V) HARD. BROWN/GRAY. **SANDY SILT**. AND CLAY. TRACE SHALE FRAGMENTS, TRACE GRAVEL, DAMP 3 7 100 SS-2B 4.25 37 23 9 10 8 36 15 12 A-4a (8) THE STATE OF THE S SS-3A A-4a (V) 1138.7 N 050 MEDIUM DENSE, BROWN, FINE SAND, TRACE 5 GRAVEL, TRACE SILT, DAMP ₩ > ₩ 14 100 SS-3B A-3 (V) 1 L FS 400 6 (VS) SS-4A A-3 (V) 1137.0 10 727 VERY STIFF, GRAY, SILT AND CLAY, LITTLE SAND. 21 100 10 1 2 2 TRACE GRAVEL, DAMP SS-4B 2.75 A-6a (V) A L 1136.0 NOTES: NONE ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 94+69, 10' RT. B-018-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1131.7 (NAVD88)EOB: 1 OF 1 START: 12/14/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 711936.3950 N, 2314269.4880 E 12/14/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1131.7 ASPHALT - 6 INCHES 1131.2 **CONCRETE - 11 INCHES** - 1 -1130.3 LOOSE, BROWN, FINE SAND, TRACE SILT, MOIST SS-1A 12 A-3 (V) 2 1129.5 18 100 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, 10 TRACE GRAVEL, TRACE SHALE FRAGMENTS, DAMP SS-1B 6 8 13 25 48 28 17 11 14 A-6a (8) 260 3 @3.5': HARD 12 33 100 SS-2 4 10 19 26 41 28 17 11 13 A-6a (7) THE PARTY OF THE P N 050 5 46 100 SS-3 12 A-6a (V) @5.2': GRAY, LITTLE SANDSTONE FRAGMENTS 2011A 400 6 WBAN @6': LITTLE GRAVEL 16 21 52 100 SS-4 12 A-6a (V) 26 1124.2

NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 133+59, 9' RT. B-028-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1118.3 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: COORD: 715762.6270 N, 2313581.7020 E 12/19/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1118.3 ASPHALT - 6 INCHES 1117.8 **CONCRETE - 10 INCHES** - 1 -1117.0 BROWN, FINE SAND, TRACE SILT SS-1A A-3 (V) 1116.5 MEDIUM STIFF, BROWN, SILT AND CLAY, SOME 2 SAND, TRACE GRAVEL, MOIST 2 8 100 SS-1B 7 8 14 21 50 27 16 11 17 A-6a (8) 260 3 1115.1 SS-2A A-6a (V) --MEDIUM DENSE. BROWN. GRAVEL AND STONE FRAGMENTS WITH SAND AND SILT, TRACE CLAY, DAMP 12 26 100 SS-2B 27 39 21 9 4 19 17 2 A-2-4 (0) 4 THE PARTY OF THE P D 000 @4.8': GRAY/BROWN 5 12 - P 13 100 SS-3 5 A-2-4 (V) 2011A 400 1112.3 6 WBAN DENSE, BROWN, FINE SAND, LITTLE GRAVEL. TRACE SILT, DAMP 100 SS-4 17 41 A-3 (V) 20 1110.8 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 137+44, 12' LT. B-029-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1116.3 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/19/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 716139.7470 N, 2313501.4920 E 12/19/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm (%) (tsf) GR CS FS SI CL LL PL PΙ WC **AND NOTES** ID 1116.3 ASPHALT - 9 INCHES 1115.5 AGGREGATE BASE - 19 INCHES - 1 -WSA TO SS-1A A-1-b (V) 2 1114.0 14 100 STIFF, GRAY, **SANDY SILT**, SOME CLAY, TRACE GRAVEL, TRACE SHALE FRAGMENTS, DAMP SS-1B 16 20 42 21 24 15 9 A-4a (6) 14 1113.3 3 MEDIUM STIFF, GRAY, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, MOIST 3 100 SS-2 8 4 13 | 21 54 30 18 12 19 A-6a (9) 260 attitiko. @4.5': STIFF, BROWN/GRAY 1.75 5 SS-3A A-6a (V) N 900 22 100 10 1110.8 ₹> ÞZ VERY STIFF, GRAY, SILT, SOME CLAY, TRACE 1 L SS-3B 3.50 17 A-4b (V) GRAVEL, TRACE SAND, DAMP 400 6 @6': MOIST William) 10 29 100 SS-4 13 A-4b (V) 13 1 2 1108.8 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

PROJECTS\2279801.GPJ

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 141+29, 28' RT. B-030-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1111.7 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: COORD: 716525.6900 N, 2313481.4950 E 12/19/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1111.7 ASPHALT - 7.5 INCHES 1111.1 **CONCRETE - 9.5 INCHES** - 1 -1110.3 BROWN. FINE SAND. TRACE SILT 1 2 2 2 1110.0 SS-1A A-3 (V) MEDIUM STIFF, GRAY/BROWN, SANDY SILT, AND 2 CLAY, TRACE GRAVEL, DAMP 2 6 100 A-4a (8) SS-1B 8 3 8 43 38 27 17 10 15 250 3 SS-2A A-4a (V) 1108.4 VERY STIFF, GRAY/BROWN, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, TRACE SHALE FRAGMENTS, 5 17 100 DAMP SS-2B 3.25 2 6 10 22 60 31 | 17 14 A-6a (10) 15 10 4 400p @4.5': BROWN/GRAY, MOIST N OSD 5 22 56 SS-3 3.75 20 A-6a (V) 10 1 L 400 6 (VS) @6.1': DAMP SS-4 11 24 100 15 A-6a (V) 1 2 2 11 A L 1104.2 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 145+13, 17' LT. B-031-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1105.7 (NAVD88)EOB: 1 OF 1 START: 12/19/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 716898.1090 N, 2313378.1170 E 12/19/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1105.7 ASPHALT - 7 INCHES 1105.1 **CONCRETE - 9 INCHES** - 1 -1104.4 LOOSE, BROWN, FINE SAND, TRACE SILT, DAMP SS-1A 140 A-3 (V) 1103.9 STIFF, BROWN, SANDY SILT, LITTLE GRAVEL, LITTLE 2 CLAY, DAMP 61 SS-1B 15 28 9 37 11 20 16 4 12 A-4a (3) 1102.7 3 VERY STIFF, BROWN/GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, MOIST 13 100 SS-2 2.75 1 6 20 22 51 31 | 19 12 20 A-6a (8) 260 @4.6': GRAY/BROWN. SOME SANDSTONE NOD FRAGMENTS, DAMP 5 14 100 SS-3 18 A-6a (V) 4 6 (VS) 9 100 SS-4 20 3.25 16 A-6a (V) 1 2 2 @6.8': BROWN A L 1098.2 NOTES: NONE

EXPLORATION ID DRILLING FIRM / OPERATOR: PROJECT: GEA/LAK-44-18.45/00.00 TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 149+01, 10' RT. B-032-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1100.2 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: COORD: 717285.9060 N, 2313344.3620 E 12/19/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL ы WC **AND NOTES** ID 1100.2 ASPHALT - 5.5 INCHES 1099.8 **CONCRETE - 9 INCHES** 1 1099.0 BROWN, FINE SAND, TRACE SILT 1098.6 SS-1A - A A-3 (V) HARD, GRAY/BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL. MOIST 2 3 100 SS-1B 7 27 40 17 4.25 1 25 30 13 17 A-6a (8) 240 1097.2 3 HARD. GRAY/BROWN. SILTY CLAY. AND SAND. MOIST 13 100 SS-2 4.50 0 10 26 29 35 34 17 17 19 A-6b (9) ARIA DO N 950 @4.5': VERY STIFF SS-3A 3.75 13 A-6b (V) 1095.2 5 3 > M MEDIUM DENSE, BROWN, COARSE AND FINE SAND, 21 100 1 L LITTLE CLAY, LITTLE GRAVEL, TRACE SILT, MOIST SS-3B A-3a (V) 400 14 1094.2 (FEAN) 6 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND. TRACE GRAVEL, TRACE SHALE FRAGMENTS, DAMP 1 2 2 2 11 100 SS-4 24 3.50 16 A-6a (V) 11 11 1092.7 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

CTS\2279801.GPJ

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 187+86, 11' RT. B-042-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1058.7 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/20/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 721124.4230 N, 2312745.6970 E 12/20/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1058.7 ASPHALT - 8 INCHES 1058.0 AGGREGATE BASE - 7 INCHES 1 · 1057.4 MEDIUM DENSE, BROWN, GRAVEL AND/OR STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, 1 7 7 DAMP (CRUSHED STONE) FILL 2 18 100 SS-1 15 39 32 11 3 NP NP NP A-1-b (0) <100 10 3 @3': LOOSE, TRACE SILT NP NP 3 7 100 SS-2 12 42 37 7 2 NP A-1-b (0) N 050 5 SS-3A A-1-b (V) 100 ₩ > ₩ 1053.2 **w** 1053.2 1 L SOFT, RED/BROWN, SILTY CLAY, LITTLE CRUSHED SS-3B 15 A-6b (V) 400 STONE. LITTLE BRICK FRAGMENTS. DAMP FILL 6 1052.5 SS-4A A-6b (V) WBAN HARD, BROWN, SILT AND CLAY, SOME SAND, LITTLE CRUSHED STONE. TRACE BRICK FRAGMENTS. DAMP ا ح لد FILL 36 14 100 SS-4B 4.50 16 A-6a (V) 1 7 19 A L 1051.2 NOTES: NONE ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 191+74, 14' LT. B-043-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1054.2 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/20/22 END: SAMPLING METHOD: COORD: 721503.9850 N, 2312662.0500 E 12/20/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1054.2 ASPHALT - 7 INCHES 1053.6 **CONCRETE - 8 INCHES** - 1 -1052.9 LOOSE, BROWN, FINE SAND, TRACE SILT, MOIST SS-1A 12 A-3 (V) 1052.3 VERY STIFF, BROWN, SANDY SILT, LITTLE 2 SANDSTONE FRAGMENTS, TRACE CLAY, TRACE 22 100 SHALE FRAGMENTS. DAMP 14 27 15 SS-1B 19 42 4 20 5 A-4a (2) 250 3 @3.3': HARD. TRACE SANDSTONE FRAGMENTS 8 22 100 SS-2 4.50 14 A-4a (V) 4 1049.7 ALL DE HARD, GRAY/BROWN, **SANDY SILT**, "AND" CLAY, TRACE GRAVEL, DAMP N OSD 5 ~ > P 44 100 SS-3 10 16 36 37 28 18 10 15 A-4a (8) 1 2 L 400 1048.2 6 HARD, GRAY, SILT AND CLAY, LITTLE GRAVEL. (VS) LITTLE SAND, DAMP 20 20 100 SS-4 12 4.50 12 A-6a (V) 1 2 2 A L 1046.7 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

-S\2279801.GPJ

	DRILLING I SAMPLING			3.5" S	PT	SPT/ RQD	ENE	RGY R	ON DATE: ATIO (%): SAMPLE ID	HP	66	GRAD	COO	RD:	)		30.24	60 N, RG	EOB: , 2312 WC	7.5 ft. 630.4430 E ODOT CLASS (GI)	SO4 ppm
DESCRIPTION NOTES		METHO	ELEV. 1046.8 1046.4				N	REC	SAMPLE	HP	(		ATIO	N (%)	)	ATT	ERBE	RG		ODOT	SO4
NOTES			1046.8	DEPT	THS		N <sub>60</sub>												WC		
ESILT			1046.4			-		(11)		(,											
					-	-															
			1045.7																		
			10-10.7		<b>⊢</b> 1 −																
T, LITTLE CLAY,		I F S	<b>!</b>		'																
, LITTLE OLAT,	TDACE		1045.2						SS-1A	<del>  -  </del>			-							A-3 (V)	-
	TIVACE				_ 2 -	3 4 8	13	100	SS-1B	-	1	18	21	44	16	24	17	7	14	A-4a (5)	-
			1043.8		2				SS-1C	-	-	-	-	-	-	-	-	-	-	A-4a (V)	130
, LITTLE CLAY,	TRACE								SS-2A	4.50	-	-	-	-	-	-	-	-	13	A-4a (V)	-
					_ 4 -	6 8 11	21	100	SS-2B	-	8	12	22	43	15	25	18	7	12	A-4a (5)	250
			1042.1		-				SS-3A	-	-	-	-	-	-	-	-	-	-	A-4a (V)	-
	TTLE	++++	+ + + +		- 5 -	6	26	100	SS-3B	3.00	-	-	-	-	-	-	-	-	15	A-4b (V)	-
FRAGMENTS. L	TTLE	+++-	+ + + + +		_ 6 -			100	SS-3C	-	-	-	-	-	-	-	-	-	-	A-4b (V)	-
		+++-	+ + + + + + + + + + + + + + + + + + +		- - 7 -	9 9 9	20	100	SS-4	-	-	-	-	-	-	-	-	-	13	A-4b (V)	-
		+++-	1039.3	FOB-																	
	SOME CLAY, LI MENTS, DAMP	SOME CLAY, LITTLE MENTS, DAMP FRAGMENTS. LITTLE	SOME CLAY, LITTLE :::: MENTS, DAMP ::::	T, LITTLE CLAY, TRACE  1042.1  SOME CLAY, LITTLE  MENTS, DAMP  FRAGMENTS. LITTLE	SOME CLAY, LITTLE MENTS, DAMP FRAGMENTS. LITTLE	T, LITTLE CLAY, TRACE    1042.1     1042.1	T, LITTLE CLAY, TRACE  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  11  1039.3	T, LITTLE CLAY, TRACE  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  1042.1  - 5  - 6  - 11  - 26  - 7  - 9  9  9  20  - 7  9  9  9  20	T, LITTLE CLAY, TRACE  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  1039 3  1042.1  - 5 - 6  11 26 100  - 7 - 9 9  20 100	T, LITTLE CLAY, TRACE  1043.8  - 4 - 81  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  5 - 6  11 26  100  SS-2B  SS-3A  SS-3A  SS-3B  SS-3C  - 7 - 9  9  9  20  100  SS-4	T, LITTLE CLAY, TRACE  1043.8  - 4 - 811  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  1039.3  1043.8  - 4 - 811  100  SS-2A	T, LITTLE CLAY, TRACE  1043.8  - 4 - 811	T, LITTLE CLAY, TRACE  1043.8  - 4 - 811  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  1039.3  1043.8  - 4 - 811  100  SS-2A	T, LITTLE CLAY, TRACE    1043.8	T, LITTLE CLAY, TRACE    1043.8	T, LITTLE CLAY, TRACE  1043.8  - 4 - 8 11 21 100	T, LITTLE CLAY, TRACE  1043.8  - 4 - 811  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  5 - 6 - 11 13  6 8 11 21 100  SS-2B - 8 12 22 43 15 25  SS-3A	T, LITTLE CLAY, TRACE  1043.8  - 4 - 8 11 21 100 SS-2B - 8 12 22 43 15 25 18  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  5 - 6 11 13 26 100 SS-3C	F, LITTLE CLAY, TRACE  1042.1  SOME CLAY, LITTLE MENTS, DAMP  FRAGMENTS. LITTLE  1039.3  SS-2A 4.50	T, LITTLE CLAY, TRACE  1043.8  - 4 - 8 11 21 100 SS-2B - 8 12 22 43 15 25 18 7 12  SOME CLAY, LITTLE  MENTS, DAMP  FRAGMENTS. LITTLE  - 6 - 11 13 26 100 SS-3B 3.00 15  SS-3C 13	T, LITTLE CLAY, TRACE    1043.8

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 199+67, 13' LT. B-045-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1039.2 (NAVD88)EOB: 1 OF 1 START: 12/21/22 END: 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 COORD: 722287.6470 N, 2312539.8630 E REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm (%) GR CS FS SI CL LL PL ы WC **AND NOTES** ID (tsf) 1039.2 ASPHALT - 12 INCHES 1038.2 - 1 -BROWN, FINE SAND, TRACE SILT 1037.9 Ø800 STIFF, BROWN, SILT AND CLAY, SOME SANDSTONE FRAGMENTS, LITTLE SAND, DAMP 2 12 100 SS-1 30 9 33 19 27 16 260 11 11 A-6a (4) 3 @3': VERY STIFF SS-2A 2.50 12 A-6a (V) 1035.9 **w** 1035.9 VERY STIFF, BROWN, **SANDY SILT**, SOME CLAY, TRACE SANDSTONE FRAGMENTS, TRACE SHALE 6 15 100 FRAGMENTS, DAMP SS-2B 3.00 11 20 45 20 24 18 17 4 A-4a (6) \$>\ 1034.5 SS-3A A-4a (V) THE PARTY OF THE P MEDIUM DENSE. BROWN. FINE SAND. TRACE SILT. 5 SS-3B 9 A-3 (V) DAMP 1033.9 18 100 J ODD 40m VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, LITTLE GRAVEL, LITTLE SHALE FRAGMENTS, DAMP ~ > D SS-3C 15 A-6a (V) 6 400 SS-4A A-6a (V) ØBAD. 13 62 100 23 1032.2 33 7/2/ SANDSTONE, BROWN/GRAY, SEVERELY TO HIGHLY 1 7 7 SS-4B 10 Rock (V) WEATHERED. 1031.7 EOB

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 203+61, 10' RT. B-046-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1030.5 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/21/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 722681.0170 N, 2312512.2300 E 12/21/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1030.5 ASPHALT - 11 INCHES 1029.6 AGGREGATE BASE - 4 INCHES 1 · 1029.2 48M MEDIUM DENSE, BROWN, COARSE AND FINE SAND, LITTLE SILT, TRACE GRAVEL, TRACE CLAY, DAMP 7 > 1 7 7 NP SS-1A 25 55 17 2 NP NP 2 10 A-3a (0) 17 100 SS-1B A-3a (V) <100 1027.5 3 LOOSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, DAMP NP NP NP 3 7 100 SS-2 7 45 40 6 2 A-1-b (0) <100 THE PARTY OF THE P SS-3A A-1-b (V) 5 N 95/2 11 100 1025.1 STIFF, BROWN/GRAY, SILT AND CLAY, SOME SAND, 7 > DZ SS-3B 13 A-6a (V) TRACE GRAVEL, DAMP 1 1 6 400 @6.3': VERY STIFF. LITTLE GRAVEL 480m 7 > L 9 100 SS-4 20 A-6a (V) 170 1023.0 NOTES: NONE

ASPHALT - 5.25 INCHE  CONCRETE - 8.5 INCHE  BROWN, FINE SAND, T  STIFF, BROWN, SILT A TRACE GRAVEL, DAMF	RIAL DESCRIPTION AND NOTES S ES RACE SILT	SAMPLING N	WETT C	ELEV. 1022.8	DEP1		SPT/ RQD					00	CME 75 TRUCK 844  CME AUTOMATIC  TION DATE: 3/15/21  RATIO (%): 66									PATION 7-0-22 PAGE 1 OF	
CONCRETE - 8.5 INCHE BROWN, FINE SAND, T STIFF, BROWN, SILT A	S ES RACE SILT			X .		1	ROD	N <sub>60</sub>	(%)	SAMPLE ID	HP (tsf)			ATIO	<del></del>			ERBE		wc	ODOT CLASS (GI)	SO4 ppm	BA
	ND CLAY SOME S		_XX	1021.7		_ 1 -			(70)		(tor)												W V V V V V V V V V V V V V V V V V V V
	ND CLAY SOME S		F.S.	1021.2		ļ.,	-			SS-1A	h - d	- /	_		_	-	_			_	A-3 (V)		_1
		AND,		1019.8		- 2 -	4 4 7	12	100	SS-1A	-	4	8				28	16	12	14	A-6a (9)	270	A A CANA
HARD, BROWN, SILT A		AND,		1019.5		<del>-</del> 3 -				SS-2A	4.50	-	-	-	-	-	-	-	-	18	A-6a (V)	-	É
TRACE GRAVEL, MOIS' VERY STIFF, BLACK/BF SAND, TRACE ORGANI	ROWN, SILT AND C	LAY, LITTLE				- 4 -	4 8 10	20	100	SS-2B	-	0	5	13	24	58	33	19	14	21	A-6a (10)	-	A STANKE AND
_@4.5': STIFF, DAMP			////	1018.1						SS-3A	1.75	•	-	-	-	-	-	-	-	18	A-6a (V)	-	2
HARD, BROWN/GRAY, LITTLE SANDSTONE FF		OME SAND,				- 5 - - - 6 -	12 18 24	46	67	SS-3B	-	-	-	-	-	-	-	-	-	12	A-6a (V)	-	A A A A A A A A A A A A A A A A A A A
				1016.3			20			SS-4A	-	•	-	-	-	-	-	-	-	-	A-6a (V)	-	<b>A</b> Q
HARD, BROWN, <b>SAND</b> GRAVEL, DAMP	<b>/ SILT</b> , LITTLE CLA	Y, TRACE		1015.3	EOB-	- 7 -	20 35 35	77	83	SS-4B	-	-	-	-	-	-	-	-	-	14	A-4a (V)	-	VI VI V
					EOB-											·							

EXPLORATION ID DRILLING FIRM / OPERATOR: PROJECT: GEA/LAK-44-18.45/00.00 TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 211+39, 9' RT. B-048-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1016.8 (NAVD88)EOB: 7.5 ft. 1 OF 1 START: 12/21/22 END: COORD: 723457.5490 N, 2312490.0160 E 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm (%) GR CS FS SI CL LL PL ы WC **AND NOTES** ID (tsf) 1016.8 ASPHALT - 7 INCHES 1016.2 **CONCRETE - 9 INCHES** 1 · 1015.5 BROWN, FINE SAND, TRACE SILT 1 2 2 2 1015.1 SS-1A A-3 (V) STIFF, BROWN, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, MOIST 2 SS-1B 1.75 12 22 31 17 61 14 22 A-6a (10) 2 100 SS-1C 160 A-6a (V) 3 SS-2A 2.00 26 A-6a (V) 1013.5 VERY STIFF, GRAY/BROWN, SILT AND CLAY, LITTLE SAND, MOIST 3 12 100 SS-2B 3.00 0 2 9 21 68 34 21 13 22 A-6a (9) 270 4 1012.3 ALL SON HARD, GRAY/BROWN, SILT AND CLAY, LITTLE SAND. SS-3A 4.50 DAMP 15 A-6a (V) NOD 5 @5.1': BROWN, SOME SAND, TRACE GRAVEL 28 100 2000 2000 SS-3B 4.50 15 A-6a (V) 400 1010.8 6 HARD. BROWN. SILT AND CLAY. SOME SAND. LITTLE (VS) GRAVEL, TRACE SHALE FRAGMENTS, DAMP 100 SS-4 18 15 A-6a (V) 1 2 2 18 A L 1009.3 NOTES: NONE

EXPLORATION ID PROJECT: DRILLING FIRM / OPERATOR: GEA/LAK-44-18.45/00.00 TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 215+31, 14' LT. B-049-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1012.6 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/21/22 END: COORD: 723849.9680 N, 2312495.1650 E 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1012.6 ASPHALT - 7 INCHES 1012.0 **CONCRETE - 8 INCHES** - 1 -1011.3 BROWN, FINE SAND, TRACE SILT 1 2 2 2 1010.9 SS-1A A-3 (V) STIFF, BROWN, SANDY SILT, SOME CLAY, TRACE GRAVEL, DAMP 2 3 100 A-4a (8) SS-1B 1 14 43 35 29 20 9 16 260 1009.6 3 STIFF. BROWN. SILT AND CLAY. SOME SAND. SS-2A 1.50 18 A-6a (V) TRACE GRAVEL, DAMP @3.5': LITTLE SAND, MOIST 3 8 100 57 SS-2B 2.25 2 5 23 33 19 A-6a (10) 4 13 14 19 GRAPA 1008.1 VERY STIFF, GRAY/BROWN, SILTY CLAY, LITTLE SAND, TRACE GRAVEL, MOIST N OSD 5 8 19 100 SS-3 2.50 22 A-6b (V) Ø >₩ 1 L 400 1006.6 6 VERY STIFF, BROWN/GRAY, SILT AND CLAY, SOME (VS) SAND, TRACE GRAVEL, MOIST 10 29 100 SS-4 12 2.50 A-6a (V) 1 2 2 14 A L 1005.1 NOTES: NONE

BEILY   DEPTHS   SPT   No.   REC   SAMPLE   HP   GRADATION (%)   ATTERBERG   NO.   CLASS (G)   SPM   BACK   CANDING STONE   1007.3   1007.1   100	PROJECT: GEA/LAK-44-18.45/00.00  TYPE: ROADWAY  PID: 114163 SFN: N/A	DRILLING FIRM / C SAMPLING FIRM / DRILLING METHOL	LOGGER: _			HAM	IMER:	CME 7 CME A	NOTUA			ALIC	SNME	ENT:			SR4	24, 27' 4 )EOB:			0-22 PAGE
TOPSOIL 2 INCHES  MEDIUM DENSE, BLACK GRAVEL AND/ORSTONE FRAGMENTS, LITTLE SLAG FRAGMENTS WITH SAND, LITTLE SLAG FRAGMENTS, LITTLE CHAPTER, MOIST (CRUSHED)  SISTER, ERACKFROWN SILT AND CLAY, LITTLE SAND, TRACE GRAY/BROWN, SILT AND CLAY.  LITTLE SAND, MOIST  BY SING 1, 15 2, 5 0, 20 0, 4 10 8 22 56 33 20 13 20 A-6a (0) 270  SSTONE, MOIST FILL  STONE, MOIST FRAGMENTS, TRACE CRUSHED  SSTONE, MOIST FRAGMENTS, TRACE SAND, MOIST FRAGMENTS, TRACE CRUSHED  SSTONE, MOIST FRAGMENTS, TRACE CR	START: <u>12/21/22</u> END: <u>12/21/22</u>	SAMPLING METHO	DD:	SPT		ENE		. ,		66		COC	ORD:	7	72423	35.92	00 N,	23125	798.7400	<u> </u>	1 OF 1
TOPSOIL - 2 INCHES AND FOLSE  TOPSOIL - 2 INCHES AND FOLSE  TOPSOIL - 2 INCHES AND FOLSE  TOPSOIL - 2 INCHES AND TRACE SILT  TOPSOIL - 3 INCHES AND TRACE SI		V	ELEV.	DEPTHS		Non							$\overline{}$		_		_				
MEDIUN DENSE, BACK GRAVEL ANDORS STONE FRAGMENTS WITH SAND, ITTIRE CHINDERS, MOIST (CRUSHED MARCH SILT) 1006.5  PROMOMENTS, LITTLE CHINDERS, MOIST (CRUSHED MARCH SILT) 1006.5  TITLE CHINDERS, MOIST (CRUSHED STONE DAMP FILL) 1006.5  TITLE CHINDERS, MOIST (CRUSHED STONE DAMP SILT) 1006.5  TITLE CHINDERS, MOIST (CRUSHED STONE ST				<u> </u>	RQD	00	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI	ppiii	
EDILIM DENSE, BROWN, FINE SAND, TRACE SILT  SIFE, GRAVBROWN, SILT YCLAY, LITTLE SAND, TRACE CRUSHED STORE, LOND FILL  SIFE, GRAVBROWN, SILT AND CLAY, LITTLE SAND, TRACE CRUSHED STONE, MOIST FILL  1004.0  VERY STIFF, GRAV/BROWN, SILT AND CLAY, LITTLE SAND, MOIST  4 5 10 100 SS-3B 2.50 0 3 8 21 68 33 20 13 20 A-6a (9) 270 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (6) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (6) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (6) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (6) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (6) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (7) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (8) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (8) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (8) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (7) C-2 SS-3B 2.50 D 3 8 21 68 33 22 11 24 A-6a (8)	MEDIUM DENSE, BLACK, <b>GRAVEL AND/OI</b> FRAGMENTS WITH SAND, LITTLE SLAG ¬ FRAGMENTS, LITTLE CINDERS, MOIST (C	8,5			6		89	SS-1A	-	-	-	-	-	-	-	-	-	11	A-1-b (V)	-	47 × 7 × 7 × 7 × 7 × 7 × 7 × 7 × 7 × 7 ×
SIFE GRAVIBROWN SILT YCLAY, LITTLE SAND, TRACE CRUSHED STONE, DMP FILL  STIFE BLACK/BROWN, SILT AND CLAY, LITTLE SAND, MOIST FILL  1004.0  SS-28 2.00 4 10 8 22 56 33 20 13 20 A-6a (9) 270 SS-28 SS-2A SS-2	$^{\prime}_{\sqcap}$ MEDIUM DENSE, BROWN, <b>FINE SAND</b> , TR		1006.0	<del>-</del> 1	1 "					- 2	- 5			- 67			- 16			-	COUTNETS <
TRACE CRUSHED STONE, DAMP FILL   1004.0   2   3   3   7   100   SS-2B   2.00   4   10   8   22   56   33   20   13   20   A-6a (9)   270   3   3   3   7   100   SS-2B   2.00   4   10   8   22   56   33   20   13   20   A-6a (9)   270   3   3   4   5   10   100   SS-3B   2.50   0   3   8   21   68   33   22   11   24   A-6a (8)   - 1   2   4   A-6a (8)   - 1   2   A-6a (8)   - 1   2   A-6a (8)   - 1   2   A-6a (8)   - 1   A-6a		E SAND	1005.5	Γ					_	-	-			-	-		-		-		William J
VERY STIFF, GRAY/DARK BROWN, SILT AND CLAY, LITTLE SAND, MOIST  - 4 - 3 4 5 10 100 SS-3B 2.50 0 3 8 21 68 33 22 11 24 A-6a (8)	\TRACE CRUSHED STONE, DAMP FILL STIFF, BLACK/BROWN, SILT AND CLAY, L SAND, TRACE SLAG FRAGMENTS, TRACE	ITTLE		_		7	100	SS-2B	2.00	4	10	8	22	56	33	20	13	20	A-6a (9)	270	1
@4.8: GRAY/BROWN    001.3   1001.3   1000.4   EOB   1000.4   EOB   1000.4   EOB   EO			1004.0					SS-3A	-	-	-	-	-	-	-	-	-	-	A-6a (V)	-	1 X X
@4.8': GRAY/BROWN    1001.3	LITTLE CAND MOIOT	ND CLAY,		- 4			100	SS-3B	2.50	0	3	8	21	68	33	22	11	24	A-6a (8)	-	TANK TO THE TANK T
VERY SIRF IO HARD, GRAY/BROWN, SANDY SILT, LITTLE CLAY, TRACE SANDSTONE FRAGMENTS, TRACE GRAVEL, DAMP  NOTES: NONE	@4.8': GRAY/BROWN		1001 3	- 5 -			100	SS-4	4.00	-	-	-	1	-	-	-	-	18	A-6a (V)	-	
	LITTLE CLAY, TRACE SANDSTONE FRAGM TRACE GRAVEL, DAMP	· · · · · · · · · · · · · · · · · · ·		_	17 50/5"	-	100	SS-5	-	-	-	-	-	-	-	-	-	14	A-4a (V)	-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		OLIANITITIES		0.140/55 ::		0.55	UTC:	TE 01 "5"													

EXPLORATION ID DRILLING FIRM / OPERATOR: PROJECT: GEA/LAK-44-18.45/00.00 TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 223+25, 23' LT. B-051-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1002.4 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/21/22 END: COORD: 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 724640.3740 N, 2312571.0770 E REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm GR CS FS SI CL LL PL PΙ WC AND NOTES (%) ID (tsf) 1002.4 **TOPSOIL - 4 INCHES** 1002.1 SS-1A A-3 (V) (180m) BROWN, FINE SAND, TRACE SILT 1001.8 × , 42 VERY STIFF, BROWN/DARK BROWN, SANDY SILT, 14 6 89 インレ SOME CLAY, LITTLE GRAVEL, TRACE ORGANICS, 100 1 32 SS-1B 2.50 10 14 8 36 28 19 9 21 A-4a (7) TRACE SHALE FRAGMENTS. MOIST @1.5': DAMP 1 / N SS-2A 2.00 16 A-4a (V) 2 1000.1 3 9 100 VERY STIFF, BROWN, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, MOIST SS-2B 3.00 5 13 26 55 35 21 21 A-6a (10) 270 1 14 Gardo 999.3 3 STIFF, ORANGE/BROWN, SILTY CLAY, LITTLE SAND, TRACE SANSTONE FRAGMENTS. DAMP 71 SS-3 4 4 8 19 65 38 22 16 19 A-6b (10) 50/2' THE STATE OF THE S 998.3 -TR-WOOD SANDSTONE. ORANGE/BROWN. SEVERELY TO HIGHLY WEATHERED, TRACE SHALE FRAGMENTS. 7 > D 100 1 L SS-4 12 Rock (V) 50/2" 5 400 W35000 6 1 2 2 25 SS-5 35 77 83 Rock (V) 35 7 L @7': BROWN, LITTLE SHALE FRAGMENTS 994.9 -EOB-NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 227+04, 10' RT. B-052-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION: 998.5 (NAVD88) EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/21/22 END: SAMPLING METHOD: COORD: 725014.1520 N, 2312644.1810 E 12/21/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 998.5 ASPHALT - 7 INCHES 997.9 **CONCRETE - 8 INCHES** 1 · 997.2 BROWN, FINE SAND, TRACE SILT 996.8 SS-1A A-3 (V) VERY STIFF, BROWN/GRAY, SILT AND CLAY, SOME SAND, DAMP 2 SS-1B 3.50 6 14 27 53 33 20 0 13 16 A-6a (9) 12 89 SS-1C A-6a (V) 160 3 @3': GRAY/BROWN. LITTLE SAND. TRACE SANDSTONE FRAGMENTS 2279801.GPJ 17 100 SS-2 8 6 19 59 31 | 19 12 17 A-6a (9) 250 994.0 GUZDO. **SANDSTONE. BROWN. SEVERELY TO HIGHLY** WEATHERED, TRACE SHALE FRAGMENTS. N 000 100 SS-3 9 Rock (V) 50/3" 2000 2000 400 992.4 6 100/1" / - / 100/ SS-4

NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 231+11, 10' LT. B-053-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION: 993.7 (NAVD88) EOB: 1 OF 1 START: 12/21/22 END: COORD: 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 725420.4560 N, 2312667.9170 E ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG **MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 993.7 ASPHALT - 5.5 INCHES 993.2 **CONCRETE - 8.5 INCHES** - 1 -992.5 BROWN, FINE SAND, TRACE SILT 992.1 SS-1A - . . -A-3 (V) STIFF, BROWN/GRAY, SILT AND CLAY, LITTLE SAND, Z LXD TRACE ORGANICS, MOIST 2 2 6 100 SS-1B 65 20 1.50 0 4 8 23 34 14 24 A-6a (10) 270 990.7 3 VERY STIFF, BROWN, SILTY CLAY, LITTLE GRAVEL. LITTLE SAND, TRACE SANDSTONE FRAGMENTS, DAMP 8 19 100 SS-2 3.00 14 5 10 18 53 32 19 13 16 A-6a (8) THE PARTY OF THE P @4.5': BROWN/GRAY, TRACE WOOD J 050 5 × > 200 13 100 SS-3 3.50 23 A-6b (V) 2011A 400 6 @5.9': TRACE GRAVEL WBAN 16 29 100 SS-4 14 26 A-6b (V) 12 986.2 -EOB NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 102+18, 10' LT. B-060-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: **GIRDLED RD** PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1006.2 (NAVD88)EOB: 7.5 ft. 1 OF 1 START: 12/22/22 END: COORD: 724042.1640 N, 2312421.6920 E 12/22/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1006.2 ASPHALT - 6.75 INCHES 1005.6 AGGREGATE BASE - 8.25 INCHES - 1 -1004.9 BROWN, FINE SAND, TRACE SILT FILL 1 2 2 2 1004.5 SS-1A A-3 (V) VERY STIFF, BROWN, **SANDY SILT**, LITTLE CLAY, TRACE CRUSHED STONE, DAMP FILL 2 15 22 100 SS-1B 15 21 40 15 24 16 8 14 A-4a (4) 260 10 1003.2 3 VERY STIFF. BLACK/BROWN. SILTY CLAY. LITTLE CRUSHED STONE, LITTLE ASPHALT FRAGMENTS, 100 SS-2 12 A-6b (V) 1002.7 50/2" LITTLE SAND, DAMP FILL **BURIED ASPHALT - 6 INCHES** 1002.2 4 VERY STIFF, BROWN/GRAY, SILTY CLAY, SOME SAND, LITTLE GRAVEL, TRACE SANDSTONE THE STATE OF THE S FRAGMENTS, MOIST NOD 5 P > PZ 11 100 SS-3 2.25 11 6 15 21 47 34 17 17 20 A-6b (9) 1 L 400 1000.2 6 HARD. BROWN. SANDY SILT. LITLE CLAY. TRACE 480m GRAVEL, DAMP 10 24 100 SS-4 4.50 15 A-4a (V) 1 2 2 12 7 A L 998.7 NOTES: NONE ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

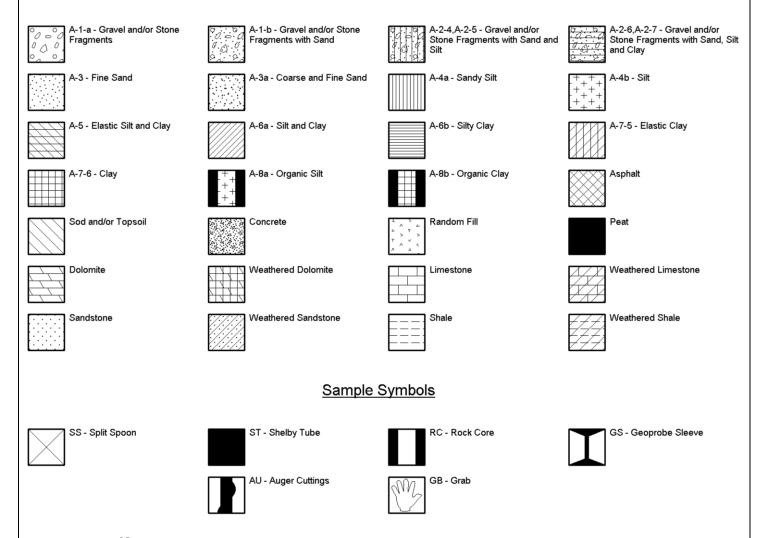
EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 104+56, 15' LT. B-061-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: **GIRDLED RD PAGE** ELEVATION:1008.5 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/21/22 END: COORD: 724142.7340 N, 2312637.1760 E 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 REC SAMPLE HP **GRADATION (%)** ATTERBERG **MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1008.5 ASPHALT - 3.75 INCHES 1008.2 **CONCRETE - 8.5 INCHES** 1007.5 - 1 -BROWN, FINE SAND, TRACE SILT FILL 1007.1 LOOSE, BROWN/GRAY, GRAVEL AND/OR STONE A L FRAGMENTS WITH SAND AND SILT, TRACE CLAY, DAMP (CRUSHED STONE) FILL 2 SS-1A 33 17 16 30 4 19 17 2 A-2-4 (0) 270 19 89 13 1005.9 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, SS-1B A-6a (V) TRACE SANDSTONE FRAGMENTS, TRACE GRAVEL, 3 DAMP SS-2A 4.50 15 A-6a (V) 6 21 100 @3.8': BROWN/GRAY, SOME SANDSTONE 13 THE STATE OF THE S FRAGMENTS, LITTLE SAND SS-2B 4.50 A-6a (V) 15 NOD ₹>₩ 5 1 1 53 100 SS-3 21 8 6 16 49 33 20 13 13 A-6a (7) Acord. Wishn. 6 1 0 N 18 26 62 100 SS-4 4.00 16 A-6a (V) 30 1001.0 -EOB NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 108+46, 9' LT. B-063-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: GIRDLED RD **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1014.8 (NAVD88)EOB: 1 OF 1 START: 12/21/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 724295.2480 N, 2312996.3880 E 12/21/22 SPT 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1014.8 ASPHALT - 6 INCHES 1014.3 AGGREGATE BASE - 7 INCHES 48000 1013.7 - 1 A-1-b (V) VERY STIFF, GRAY/BLACK, **SANDY SILT**, SOME 72/ CRUSHED STONE, LITTLE ASPHALT FRAGMENTS, 1 7 7 30 TRACE CLAY, DAMP FILL 18 100 SS-1B 33 | 16 11 36 4 28 19 A-4a (1) 270 2 1012.1 VERY STIFF, BROWN/GRAY, SILT AND CLAY, LITTLE 3 SANDSTONE FRAGMENTS. LITTLE SAND. WET A CONTRACTOR SS-2A 2.75 42 A-6a (V) 1010.8 -TR-SANDSTONE, BROWN, SEVERELY TO HIGHLY 24 93 50/3 CHARAS WEATHERED. SS-2B 10 Rock (V) NOD 5 T L 1009.2 -EOB-

NOTES: AUGER REFUSAL AT 5.6 FT.

### **LEGEND KEY**

#### Ohio Department of Transportation Soil Symbols



#### Notes:

- 1. Pavement cores and exploratory borings were performed during the period from December 12 through 22, 2022. Cores were obtained using a nominal 4-inch diameter core barrel. Borings were advanced using 3½-inch diameter solid-stem augers.
- 2. These logs are subject to the limitations, conclusions, and recommendations in the report and should not be interpreted separate from the report.
- 3. The boring locations were established in the field by TTL Associates, Inc. in general accordance with plans provided with the proposal for this project.



## **GRAIN SIZE DISTRIBUTION**



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-015-0-22

B-016-0-22

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3.0

1.0

1.132

1.274

0.01

0.006

0.005

4

8

15

8

10

14

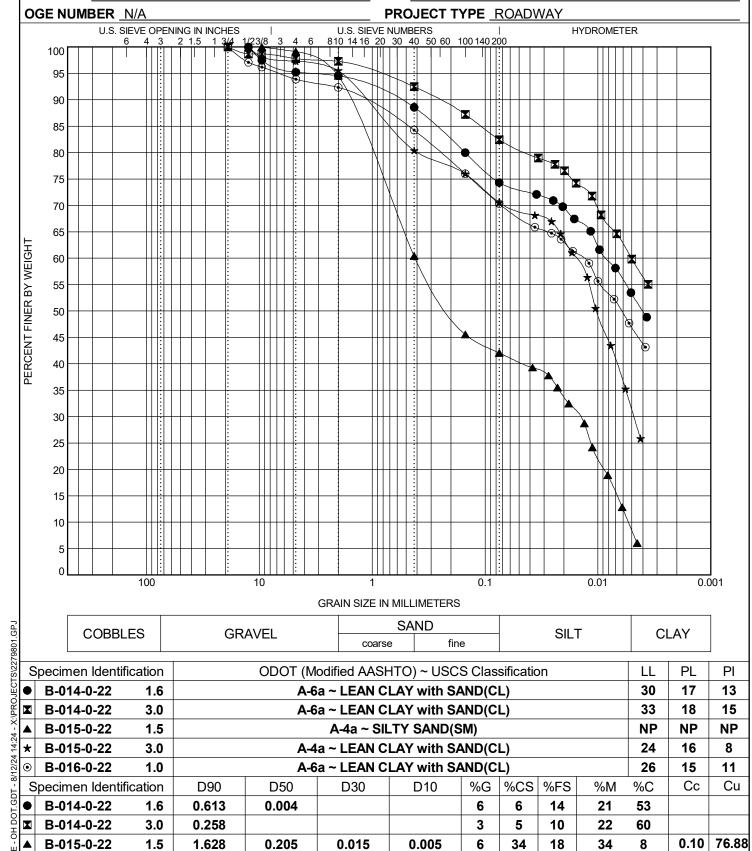
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23

31

47

**PID** 114163

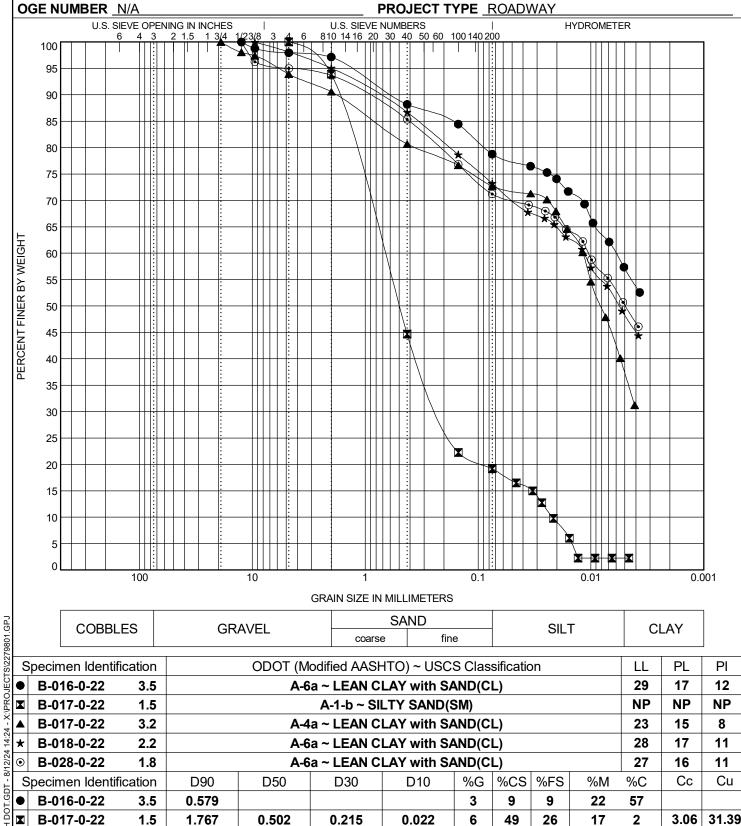


## **GRAIN SIZE DISTRIBUTION**



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



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36

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21

37

48

50

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B-017-0-22

B-018-0-22

B-028-0-22

3.2

2.2

1.8

1.826

0.781

1.006

0.008

0.006

0.005



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-030-0-22

B-030-0-22

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1.7

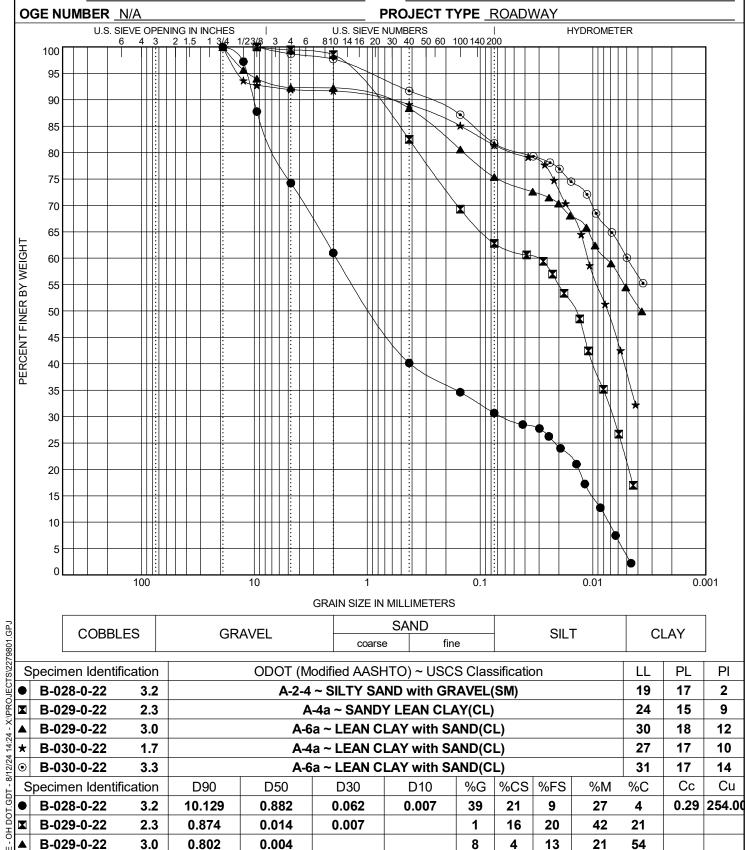
3.3

0.714

0.287

0.007

**PID** 114163



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38

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32

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15

0.061

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PROJECT <u>GEA/LAK-44-18.45/00.00</u>

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B-042-0-22

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0.464

3.349

3.0

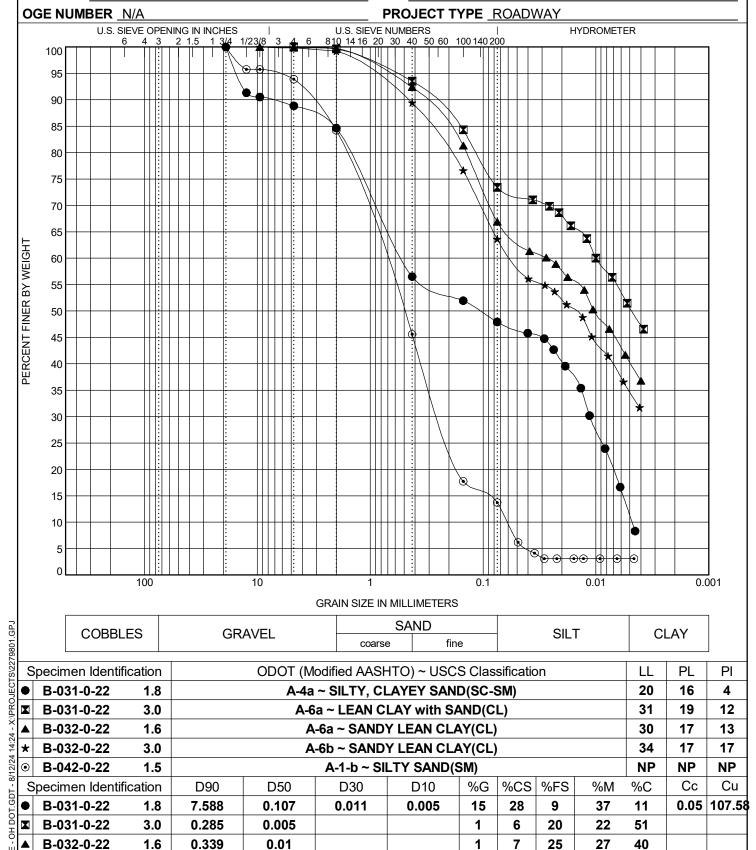
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0.015

0.507

0.237

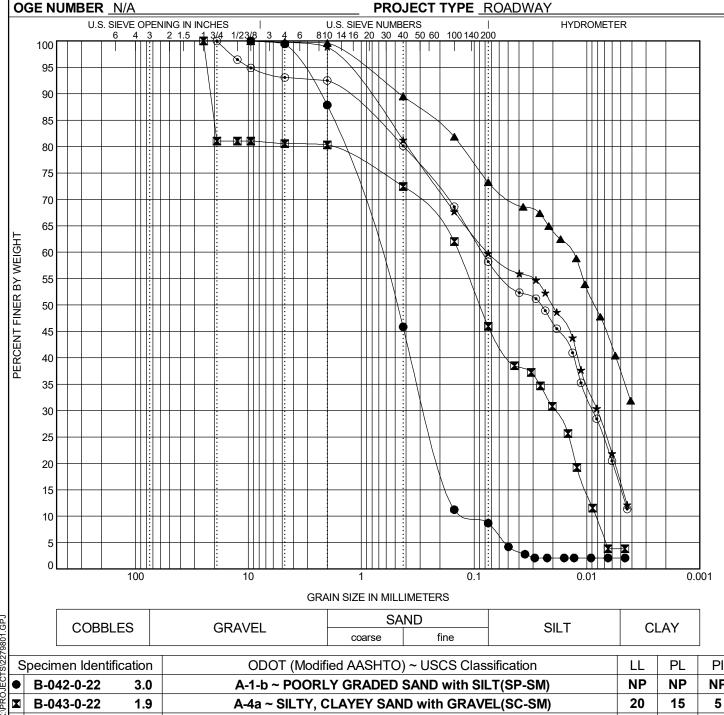
**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



٠L													
22/6	Specimen Identification	ion		ODOT (Modified AASHTO) ~ USCS Classification			LL	PL	PI				
	● B-042-0-22 3	3.0		A-1-b ~ POORLY GRADED SAND with SILT(SP-SM)			NP	NP	NP				
֓֞֝֝֟֝֝֟֝֝֟֝֝֟֝֟֝֓֓֓֓֓֓֓֓֓֓֓֓֟֟	<b>▼</b> B-043-0-22 1	1.9		A-4a ~ SILTY, CLAYEY SAND with GRAVEL(SC-SM)			20	15	5				
<u>`</u>	▲ B-043-0-22 4	4.5		A-4a ~ LEAN CLAY with SAND(CL)				28	18	10			
	★ B-044-0-22 1	1.6		A-4a ~ SANDY SILTY CLAY(CL-ML)				24	17	7			
7/7	<b>⊙</b> B-044-0-22 3	3.3	A-4a ~ SANDY SILTY CLAY(CL-ML)				25	18	7				
-	Specimen Identification	ion	D90	D50	D30	D10	%G	%CS	%FS	%M	%C	Сс	Cu
5 .	● B-042-0-22 3	3.0	2.343	0.495	0.264	0.107	12	42	37	7	2	0.91	6.70
	<b>▼</b> B-043-0-22 1	1.9	21.628	0.089	0.019	0.008	19	8	27	42	4	0.32	16.45
5	▲ B-043-0-22 4	4.5	0.457	0.009			1	10	16	36	37		
ōΙ	+	1.6	0.915	0.02	0.008		1	18	21	44	16		
3	© B-044-0-22 3	3.3	1.457	0.026	0.009		8	12	22	43	15		

GRAIN SIZE - OH DOT.GDT - 8/12/24 14:24 - X:\PROJECTS\2279801.GPJ



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-046-0-22

B-047-0-22

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1.754

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3.0

1.6

0.444

0.005

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0.115

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4

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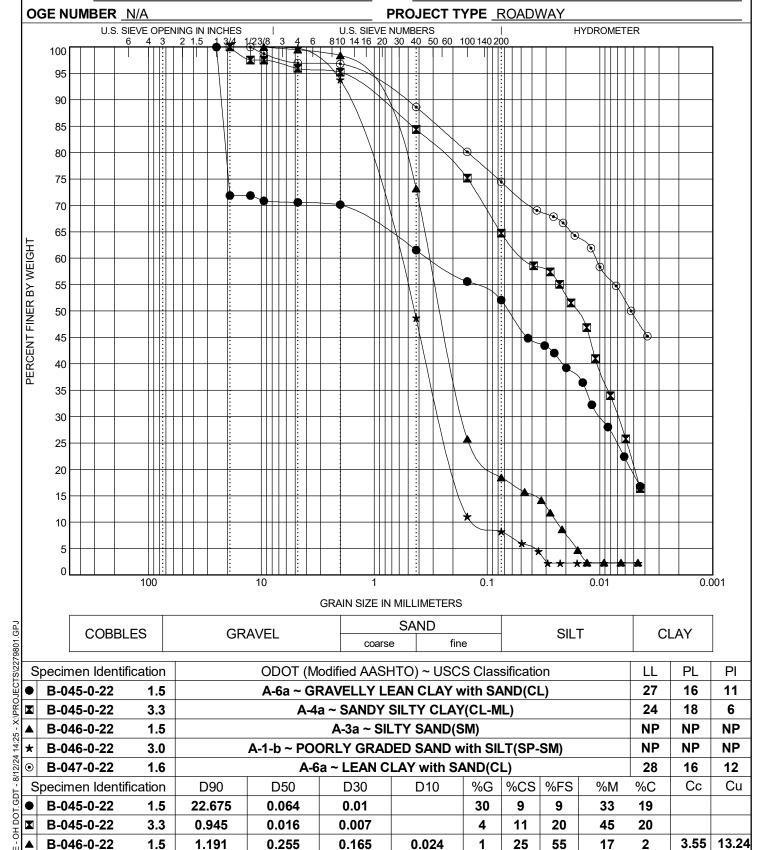
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5.44

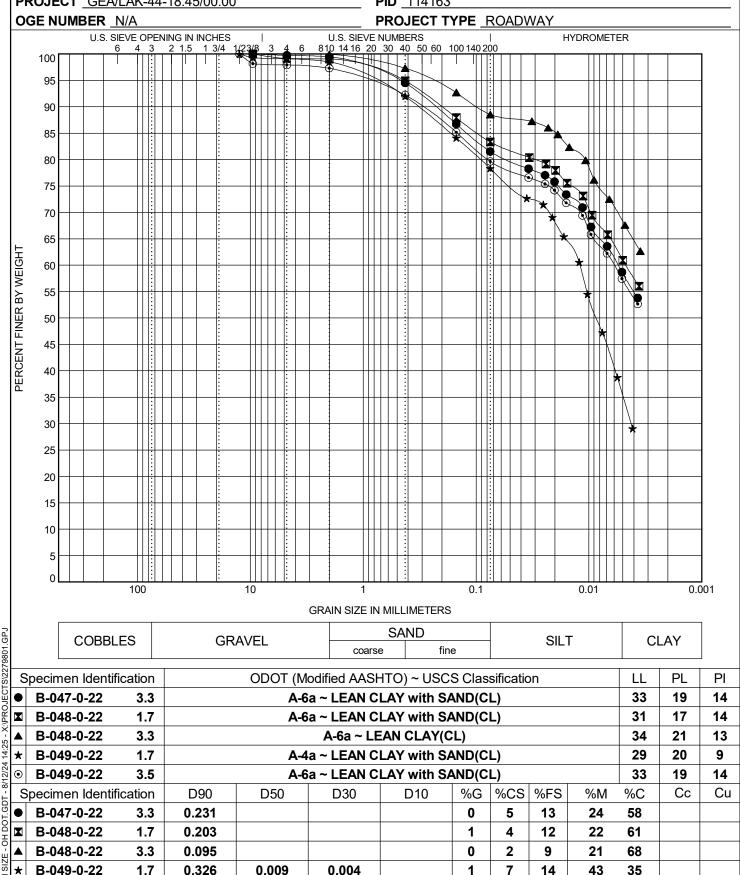
**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



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IECTS\2279801.GP

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B-049-0-22

3.5

0.305



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-051-0-22

B-051-0-22

•

0.6

2.3

1.869

0.213

0.01

0.005

10

1

14

5

8

13

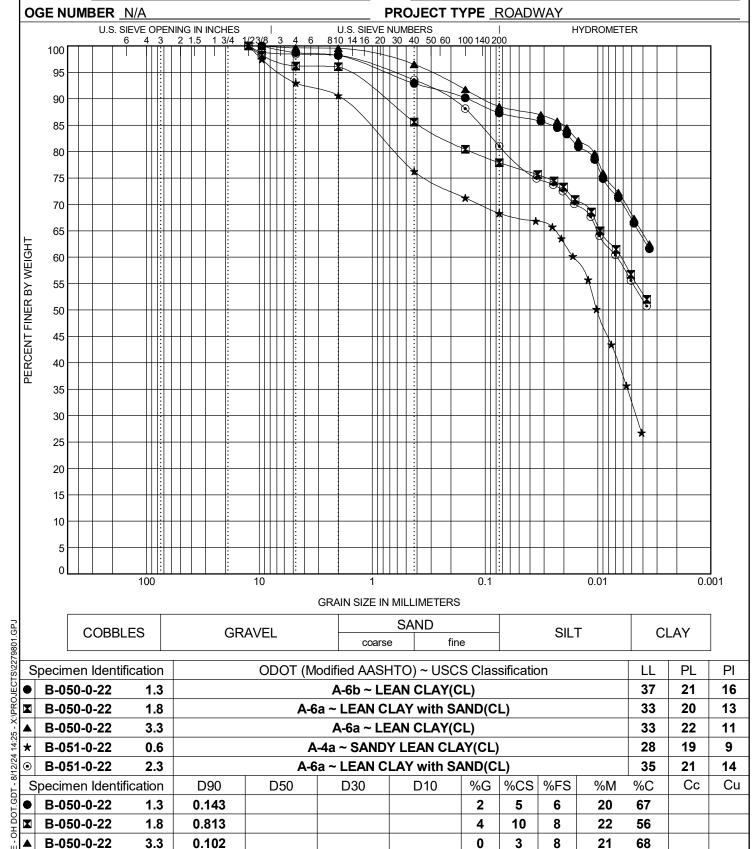
36

26

32

55

**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-053-0-22

B-053-0-22

•

1.6

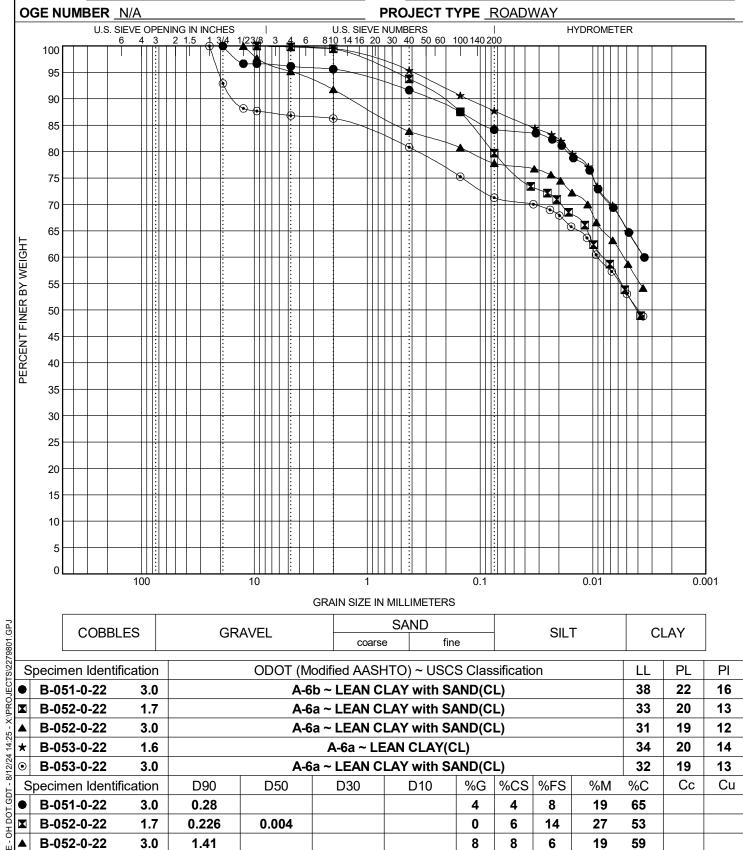
3.0

0.128

14.669

0.004

**PID** 114163



4

5

8

10

0

14

23

18

65

53

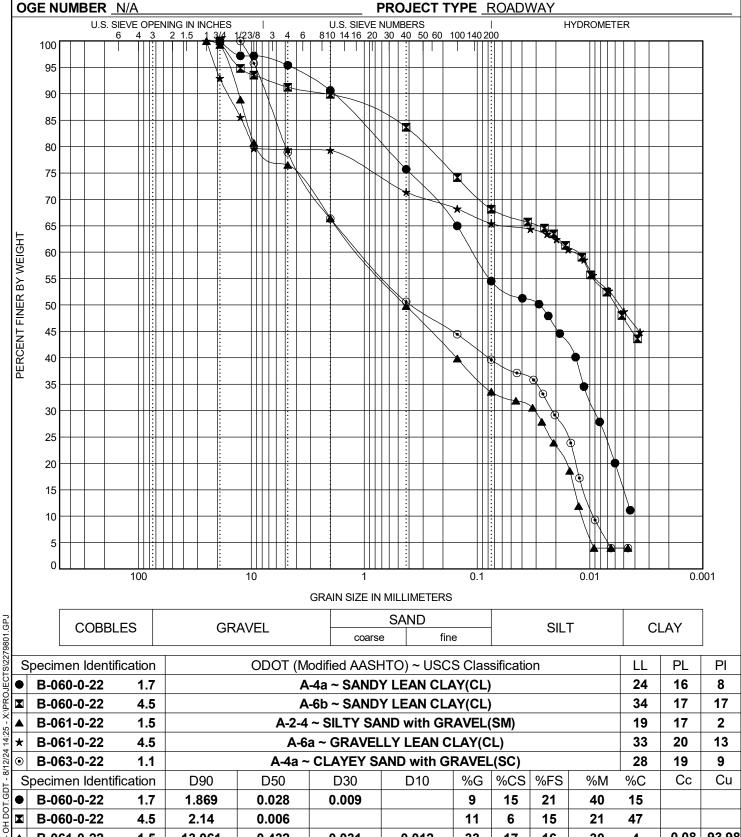
0.08 93.98

0.05 115.37



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



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B-061-0-22

B-061-0-22

B-063-0-22

1.5

4.5

1.1

13.061

16.054

7.492

0.432

0.006

0.383

0.031

0.022

0.012

0.009

33

21

33

17

8

16

16

6

11

30

16

36

4

49

4



## **Core Log For X-016-1-22**

Project: Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 14, 2022





ASPHALT THICKNESS (in.)	=	6.25
CONCRETE THICKNESS (in.)	=	8
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at 2.25 and 4 inches below top of concrete



## **Core Log For X-016-2-22**

Project: Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 14, 2022



ASPHALT THICKNESS (in.)	=	4.25
CONCRETE THICKNESS (in.)	=	9.25
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.		
Rebar encountered at approximately 5 inches below top of concrete		



## **Core Log For X-030-1-22**

Project: Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 19, 2022





ASPHALT THICKNESS (in.)	=	4.5
CONCRETE THICKNESS (in.)	=	8.25
SAND BASE THICKNESS (in.)	=	8
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at approximately 3.75 inches below top of concrete



### **Core Log For B-032-0-22**

Project: Proposed Intersection Improvements

Project Location:Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 19, 2022





ASPHALT THICKNESS (in.)	=	5.5
CONCRETE THICKNESS (in.)	=	9
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement cores appeared in good condition.

Rebar encountered at approximately 5.5 inches below top of concrete



# Core Log For B-044-0-22

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 21, 2022





ASPHALT THICKNESS (in.)	=	4.5
CONCRETE THICKNESS (in.)	=	9.25
SAND BASE THICKNESS (in.)	=	6
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at approximately 4.75 inches below top of concrete



## **Core Log For X-044-1-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 21, 2022





ASPHALT THICKNESS (in.)	=	4.5
CONCRETE THICKNESS (in.)	=	8.5
SAND BASE THICKNESS (in.)	=	7
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at approximately 5.25 inches below top of concrete



## **Core Log For B-047-0-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 21, 2022



ASPHALT THICKNESS (in.)	=	5.25
CONCRETE THICKNESS (in.)	=	8.5
SAND BASE THICKNESS (in.)	=	6
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at 4.5 and 5.5 inches below top of concrete



## **Core Log For X-050-1-22**

Project: Proposed Intersection Improvements

Project Location:Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 21, 2022



ASPHALT THICKNESS (in.)	=	5.5
CONCRETE THICKNESS (in.)	=	8
SAND BASE THICKNESS (in.)	=	6
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.	
	_



## **Core Log For B-060-0-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 22, 2022



ASPHALT THICKNESS (in.)	=	6.75
AGGREGATE BASE THICKNESS (in.)	=	8.25
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

Pavement core appeared in good condition.	



## **Core Log For B-061-0-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 21, 2022



ASPHALT THICKNESS (in.)	=	3.75
CONCRETE THICKNESS (in.)	=	8.5
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

Pavement cores appeared in good condition.
Rebar encountered at 5 inches below top of concrete

### APPENDIX A

(Engineering Calculations)





### OHIO DEPARTMENT OF TRANSPORTATION

### OFFICE OF GEOTECHNICAL ENGINEERING

# PLAN SUBGRADES Geotechnical Bulletin GB1

Instructions: Enter data in the shaded cells only. (Enter state route number, project description, county, consultant's name, prepared by name, and date prepared. This information will be transferred to all other sheets. The date prepared must be entered in the appropriate cell on this sheet to remove these instructions prior to printing.)

GEA/LAK-44-18.45/00.00 114163

### **Intersection Improvements**

### TTL Associates, Inc.

Prepared By: Christopher P. lott, P.E. Date prepared: Monday, July 22, 2024

Christopher P. lott, P.E. TTL Associates, Inc. 1915 N. 12th Street Toledo, Ohio 43604 419-214-5020

ciott@ctconsultants.com

NO. OF BORINGS:

25





#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	Boring EL.	Proposed Subgrade EL	Cut Fill
1	B-014-0-22	SR44	79+05	10	LT	CME 75 Truck 844	66	1178.5	1177.2	1.3 C
2	B-015-0-22	SR44	82+95	9	RT	CME 75 Truck 844	66	1166.9	1165.6	1.3 C
3	B-016-0-22	SR44	87+11	59	RT	CME 75 Truck 844	66	1152.8	1151.5	1.3 C
4	B-017-0-22	SR44	90+74	9	LT	CME 75 Truck 844	66	1143.5	1142.2	1.3 C
5	B-018-0-22	SR44	94+69	10	RT	CME 75 Truck 844	66	1131.7	1130.4	1.3 C
6	B-028-0-22	SR44	133+59	9	RT	CME 75 Truck 844	66	1118.3	1117.0	1.3 C
7	B-029-0-22	SR44	137+44	12	LT	CME 75 Truck 844	66	1116.3	1115.0	1.3 C
8	B-030-0-22	SR44	141+29	28	RT	CME 75 Truck 844	66	1111.7	1110.4	1.3 C
9	B-031-0-22	SR44	145+13	17	LT	CME 75 Truck 844	66	1105.7	1104.4	1.3 C
10	B-032-0-22	SR44	149+01	10	RT	CME 75 Truck 844	66	1100.2	1098.9	1.3 C
11	B-042-0-22	SR44	187+86	11	RT	CME 75 Truck 844	66	1058.7	1057.4	1.3 C
12	B-043-0-22	SR44	191+74	14	LT	CME 75 Truck 844	66	1054.2	1052.9	1.3 C
13	B-044-0-22	SR44	196+00	21	RT	CME 75 Truck 844	66	1046.8	1045.5	1.3 C
14	B-045-0-22	SR44	199+67	13	LT	CME 75 Truck 844	66	1039.2	1037.9	1.3 C
15	B-046-0-22	SR44	203+61	10	RT	CME 75 Truck 844	66	1030.5	1029.2	1.3 C
16	B-047-0-22	SR44	207+53	11	LT	CME 75 Truck 844	66	1022.8	1021.7	1.1 C
17	B-048-0-22	SR44	211+39	9	RT	CME 75 Truck 844	66	1016.8	1015.5	1.3 C
18	B-049-0-22	SR44	215+31	14	LT	CME 75 Truck 844	66	1012.6	1011.3	1.3 C
19	B-050-0-22	SR44	219+24	27	RT	CME 75 Truck 844	66	1007.3	1006.0	1.3 C
20	B-051-0-22	SR44	223+25	23	LT	CME 75 Truck 844	66	1002.4	1001.1	1.3 C
21	B-052-0-22	SR44	227+04	10	RT	CME 75 Truck 844	66	998.5	997.2	1.3 C
22	B-053-0-22	SR44	231+11	10	LT	CME 75 Truck 844	66	993.7	992.4	1.3 C
23	B-060-0-22	Girdled Rd	102+18	10	LT	CME 75 Truck 844	66	1006.2	1004.9	1.3 C
24	B-061-0-22	Girdled Rd	104+56	15	LT	CME 75 Truck 844	66	1008.5	1007.2	1.3 C
25	B-063-0-22	Girdled Rd	108+46	9	LT	CME 75 Truck 844	66	1014.8	1013.5	1.3 C

2/11/2022



#	Boring	Sample	Sam De <sub>l</sub>		Subg De		Stan Penet		HP								isture	Ohio	DOT	Sulfate Content	Proble	m	Excavate and Replace (Item 204)		Recommendation (Enter depth in
"			From	То	From	То	N <sub>60</sub>	N <sub>60L</sub>	(tsf)	LL	PL	PI	% Silt	% Clay	P200	$M_{C}$	M <sub>OPT</sub>	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
1	В	SS-1	1.6	3.0	0.3	1.7	11			30	17	13	21	53	74	13	14	A-6a	9	260		N <sub>60</sub>		12''	12''
	014-0	SS-2	3.0	5.0	1.7	3.7	14		2	33	18	15	22	60	82	19	14	A-6a	10						204 Geotextile
	22	SS-3B	5.0	6.0	3.7	4.7	22									14	8	A-3a	0						
		SS-4	6.0	7.5	4.7	6.2	24	11								13	10	A-4a	8						
2	В	SS-1A	1.4	2.8	0.1	1.5	10			NP	NP	NP	34	8	42	7	11	A-4a	1			N <sub>60</sub>		12''	12''
	015-0	SS-2A	2.8	3.3	1.5	2.0	7			24	16	8	40	31	71	14	11	A-4a	7			N <sub>60</sub> & Mc			204 Geotextile
	22	SS-2B	3.3	5.0	2.0	3.7	8									12	8	A-3a	0	100					
		SS-3B	5.0	5.3	3.7	4.0	15	7									8	A-3	0						
3	В	SS-1	0.3	3.5	-1.0	2.2	14		4.5	26	15	11	23	47	70	11	14	A-6a	7	260					None
	016-0	SS-2	3.5	6.7	2.2	5.4	6		1.25	29	17	12	22	57	79	18	14	A-6a	9						
	22	SS-3B	6.7	8.5	5.4	7.2	11		2.5							16	16	A-6b							
		SS-4	8.5	10.0	7.2	8.7	26	6								14	14	A-6a							
4	В	SS-1A/1B	0.6	3.2	-0.7	1.9	17			NP	NP	NP	17	2	19	5	6	A-1-b	0	100					None
	017-0	SS-2B	3.2	4.8	1.9	3.5	7		4.25	23	15	8	26	37	63	12	10	A-4a	6			N <sub>60</sub>			
	22	SS-3B	4.8	6.5	3.5	5.2	14									5	8	A-3	0						
		SS-4B	6.5	7.5	5.2	6.2	21	7	2.75							14	14	A-6a							
5	В	SS-1A	1.4	2.2	0.1	0.9	7									12	8	A-3	0						Rework
	018-0	SS-1B	2.2	3.0	0.9	1.7	18			28	17	11	25	48	73	14	14	A-6a	8	260					
	22	SS-2	3.0	5.2	1.7	3.9	33			28	17	11	26	41	67	13	14	A-6a	7						
		SS-3	5.2	6.0	3.9	4.7	46	7								12	14	A-6a	10						
6	В	SS-1B	1.8	3.2	0.5	1.9	8			27	16	11	21	50	71	17	14	A-6a	8	260		N <sub>60</sub> & Mc		12''	12''
	028-0	SS-2B	3.2	4.8	1.9	3.5	26			19	17	2	27	4	31	7	10	A-2-4	0						204 Geotextile
	22	SS-3	4.8	6.0	3.5	4.7	13									5	10	A-2-4	0						
		SS-4	6.0	7.5	4.7	6.2	41	8								7	8	A-3	0						
7	В	SS-1A	8.0	2.3	-0.5	1.0	14									9	6	A-1-b	0						Rework
	029-0	SS-1B	2.3	3.0	1.0	1.7	14			24	15	9	42	21	63	14	10	A-4a	6			N <sub>60</sub> & Mc		12''	
	22	SS-2	3.0	4.5	1.7	3.2	8			30	18	12	21	54	75	19	14	A-6a	9	260		N <sub>60</sub> & Mc			
		SS-3A	4.5	5.5	3.2	4.2	22	8	1.75							21	14	A-6a	10						
8	В	SS-1B	1.7	3.3	0.4	2.0	6			27	17	10	43	38	81	15	12	A-4a	8	250		N <sub>60</sub> & Mc		18''	18''
	030-0	SS-2B	3.3	4.5	2.0	3.2	17		3.25	31	17	14	22	60	82	15	14	A-6a	10						204 Geotextile
	22	SS-3	4.5	6.1	3.2	4.8	22		3.75							20	14	A-6a	10						
		SS-4	6.1	7.5	4.8	6.2	24	6								15	14	A-6a	10						
9	В	SS-1A	1.3	1.8	0.0	0.5	9									9	8	A-3	0	140					12''
	031-0	SS-1B	1.8	3.0	0.5	1.7	9			20	16	4	37	11	48	12	11	A-4a	3			N <sub>60</sub>		12''	204 Geotextile
	22	SS-2	3.0	4.6	1.7	3.3	13		2.75			12	22	51	73	20	14	A-6a	8	260		N <sub>60</sub> & Mc			
		SS-3	4.6	6.8	3.3	5.5	14	9								18	14	A-6a	10			00 20			

2/11/2022

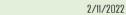


#	Boring	Sample	San De	•	~	jrade pth	le Standa Penetra		HP		Р	hysica	al Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate Content	Proble	m	Excavate ar (Item		Recommendation (Enter depth in
"			From	То	From	То	N <sub>60</sub>	N <sub>60L</sub>	(tsf)	LL	PL	PI	% Silt	% Clay	P200	$M_{\text{C}}$	M <sub>OPT</sub>	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
10	В	SS-1B	1.6	3.0	0.3	1.7	7		4.25	30	17	13	27	40	67	17	14	A-6a	8	240		N <sub>60</sub> & Mc		15''	15''
	032-0	SS-2	3.0	4.5	1.7	3.2	13		4.5	34	17	17	29	35	64	19	16	A-6b	9			N <sub>60</sub> & Mc			204 Geotextile
	22	SS-3A	4.5	5.0	3.2	3.7	18		3.75							13	16	A-6b	16						
		SS-3B	5.0	6.0	3.7	4.7	21	7								14	8	A-3a	0						
11	В	SS-1	1.3	3.0	0.0	1.7	18			NP	NP	NP	11	3	14	6	6	A-1-b	0	100					None
	042-0	SS-2	3.0	4.5	1.7	3.2	7			NP	NP	NP	7	2	9	6	6	A-1-b	0						
	22	SS-3A	4.5	5.5	3.2	4.2	4									10	6	A-1-b	0						
		SS-3B	5.5	6.2	4.2	4.9	2	2								15	16	A-6b	16						
12	В	SS-1A	1.3	1.9	0.0	0.6	9									12	8	A-3	0						Rework
	043-0	SS-1B	1.9	3.3	0.6	2.0	22			20	15	5	42	4	46	11	10	A-4a	2	250					
	22	SS-2	3.3	4.5	2.0	3.2	22		4.5							14	10	A-4a	8			Mc			
		SS-3	4.5	6.0	3.2	4.7	44	9		28	18	10	36	37	73	15	13	A-4a	8						
13	В	SS-1B/1C	1.6	3.0	0.3	1.7	13			24	17	7	44	16	60	14	12	A-4a	5	130					None
	044-0	SS-2B	3.0	4.7	1.7	3.4	21		4.5	25	18	7	43	15	58	12	13	A-4a	5	250					
	22	SS-3B	4.7	5.3	3.4	4.0	26		3							15	10	A-4b	8						
		SS-4	5.3	7.5	4.0	6.2	20	13								13	10	A-4b	8						
14	В	SS-1/2A	1.3	3.3	0.0	2.0	12		2.5	27	16	11	33	19	52	11	14	A-6a	4	260					None
	045-0	SS-2B	3.3	4.7	2.0	3.4	15		3	24	18	6	45	20	65	17	13	A-4a	6			Mc			
	22	SS-3B	4.7	5.3	3.4	4.0	18									9	8	A-3	0						
		SS-3C	5.3	7.0	4.0	5.7	18	12								15	14	A-6a	10						
15	В	SS-1A/1B	1.3	3.0	0.0	1.7	17			NP	NP	NP	17	2	19	10	8	A-3a	0	100					None
	046-0	SS-2	3.0	5.4	1.7	4.1	7			NP	NP	NP	6	2	8	6	6	A-1-b	0	100					
	22	SS-3B	5.4	6.3	4.1	5.0	11									13	14	A-6a	10						
		SS-4	6.3	7.5	5.0	6.2	20	7								14	14	A-6a	10						
16	В	SS-1B	1.6	3.0	0.5	1.9	12			28	16	12	25	49	74	14	14	A-6a	9	270					None
	047-0	SS-2A	3.0	3.3	1.9	2.2	9		4.5							18	14	A-6a	10			N <sub>60</sub> & Mc			
	22	SS-2B	3.3	4.5	2.2	3.4	20			33	19	14	24	58	82	21	14	A-6a	10			Mc			
		SS-3A	4.5	4.7	3.4	3.6	26	9	1.75							18	14	A-6a	10						
17	В	SS-1B/1C	1.7	3.0	0.4	1.7	4		1.75	31	17	14	22	61	83	22	14	A-6a	10	160		HP & Mc		24''	24''
	048-0	SS-2A	3.0	3.3	1.7	2.0	4		2							26	14	A-6a	10			N <sub>60</sub> & Mc			204 Geotextile
	22	SS-2B	3.3	4.5	2.0	3.2	12		3	34	21	13	21	68	89	22	16	A-6a	9	270		N <sub>60</sub> & Mc			
		SS-3A	4.5	5.1	3.2	3.8	24	4	4.5							15	14	A-6a	10						
18	В	SS-1B	1.7	3.0	0.4	1.7	9			29	20	9	43	35	78	16	15	A-4a	8	260		N <sub>60</sub>		12''	24''
	049-0	SS-2A	3.0	3.5	1.7	2.2	4		1.5							18	14	A-6a	10			HP & Mc			204 Geotextile
	22	SS-2B	3.5	4.5	2.2	3.2	8		2.25	33	19	14	23	57	80	19	14	A-6a	10			N <sub>60</sub> & Mc			
		SS-3	4.5	6.0	3.2	4.7	19	4	2.5							22	16	A-6b	16						

2/11/2022



#	Boring	Sample	Sam Dep		Subg De <sub>l</sub>		Stan Penet		HP		Pł	nysica	al Chara	cteristics		Mo	isture	Ohio	DOT	Sulfate Content	Proble	m	Excavate ar (Item		Recommendation (Enter depth in
			From	То	From	То	N <sub>60</sub>	N <sub>60L</sub>	(tsf)	LL	PL	PI	% Silt	% Clay	P200	$M_C$	$M_{OPT}$	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
19	В	SS-1C/2A	1.3	1.8	0.0	0.5	7		2	37	21	16	20	67	87	14	16	A-6b	10			N <sub>60</sub>		15''	15''
	050-0	SS-2B	1.8	3.3	0.5	2.0	7		2	33	20	13	22	56	78	20	15	A-6a	9	270		N <sub>60</sub> & Mc		15''	204 Geotextile
	22	SS-3B	3.3	4.8	2.0	3.5	10		2.5	33	22	11	21	68	89	24	17	A-6a	8			N <sub>60</sub> & Mc			
		SS-4	4.8	6.0	3.5	4.7	15	7	4							18	14	A-6a	10						
20	В	SS-1B/2A	0.6	2.3	-0.7	1.0	4		2	28	19	9	36	32	68	16	14	A-4a	7			N <sub>60</sub>		24''	12''
	051-0	SS-2B	2.3	3.1	1.0	1.8	9		3	35	21	14	26	55	81	21	16	A-6a	10	270		N <sub>60</sub> & Mc		12''	204 Geotextile
	22	SS-3	3.1	4.1	1.8	2.8	9			38	22	16	19	65	84	19	17	A-6b	10			N <sub>60</sub>			
		SS-4	4.1	6.0	2.8	4.7	100	4								12	0	Rock	0						
21	В	SS-1B/1C	1.7	3.0	0.4	1.7	12		3.5	33	20	13	27	53	80	16	15	A-6a	9	160					None
	052-0	SS-2	3.0	4.5	1.7	3.2	17			31	19	12	19	59	78	17	14	A-6a	9	250		Mc			
	22	SS-3	4.5	6.0	3.2	4.7	100									9	0	Rock	0						
		SS-4	6.0	6.1	4.7	4.8	100	12								5	0	Rock	0						
22	В	SS-1B	1.6	3.0	0.3	1.7	6		1.5	34	20	14	23	65	88	24	15	A-6a	10	270		HP & Mc		18''	18''
	053-0	SS-2	3.0	4.5	1.7	3.2	19		3	32	19	13	18	53	71	16	14	A-6a	8						204 Geotextile
	22	SS-3	4.5	5.9	3.2	4.6	13		3.5							23	16	A-6b	16						
		SS-4	5.9	7.5	4.6	6.2	29	6								26	16	A-6b	16						
23	В	SS-1B	1.7	3.0	0.4	1.7	22			24	16	8	40	15	55	14	11	A-4a	4	260		Mc			None
	060-0	SS-2	3.0	4.0	1.7	2.7	22									12	16	A-6b	16						
	22	SS-3	4.0	6.0	2.7	4.7	11		2.25	34	17	17	21	47	68	20	16	A-6b	9						
		SS-4	6.0	7.5	4.7	6.2	24	11	4.5							15	10	A-4a	8						
24	В	SS-1A	1.4	2.6	0.1	1.3	9			19	17	2	30	4	34	9	10	A-2-4	0	270		N <sub>60</sub>		12''	12''
	061-0	SS-2A	2.6	3.8	1.3	2.5	13		4.5							15	14	A-6a	10						204 Geotextile
	22	SS-2B	3.8	4.5	2.5	3.2	29		4.5							15	14	A-6a	10						
		SS-3	4.5	6.0	3.2	4.7	53	9		33	20	13	16	49	65	13	15	A-6a	7						
25	В	SS-1B	1.1	2.7	-0.2	1.4	18			28	19	9	36	4	40	13	14	A-4a	1	270					None
	063-0	SS-2A	2.7	4.0	1.4	2.7	18		2.75							42	14	A-6a	10			Mc			
	22	SS-2B	4.0	5.6	2.7	4.3	100									10	0	Rock	0						
								18																	





PID: 114163

County-Route-Section: GEA/LAK-44-18.45/00.00

No. of Borings: 25

Geotechnical Consultant: TTL Associates, Inc.

Prepared By: Christopher P. lott, P.E.

Date prepared: 7/22/2024

(	Chemical Stabilization Option	ıs
320	Rubblize & Roll	No
206	Cement Stabilization	Option
	Lime Stabilization	No
206	Depth	14''

Excavate and Replain Stabilization Option	
Global Geotextile Average(N60L): Average(HP):	12'' 0''
Global Geogrid Average(N60L): Average(HP):	0'' 0''

Design CBR	7
---------------	---

% Sampl	es within	6 feet of subgr	ade
N <sub>60</sub> ≤ 5	6%	HP ≤ 0.5	0%
N <sub>60</sub> < 12	36%	0.5 < HP ≤ 1	0%
12 ≤ N <sub>60</sub> < 15	16%	1 < HP ≤ 2	11%
N <sub>60</sub> ≥ 20	32%	HP > 2	32%
M+	24%		
Rock	2%		
Unsuitable	6%		

Excavate and Repl at Surface	ace
Average	16''
Maximum	24''
Minimum	12"

% Proposed Subgrade Su	ırface
Unstable & Unsuitable	52%
Unstable	52%
Unsuitable	0%

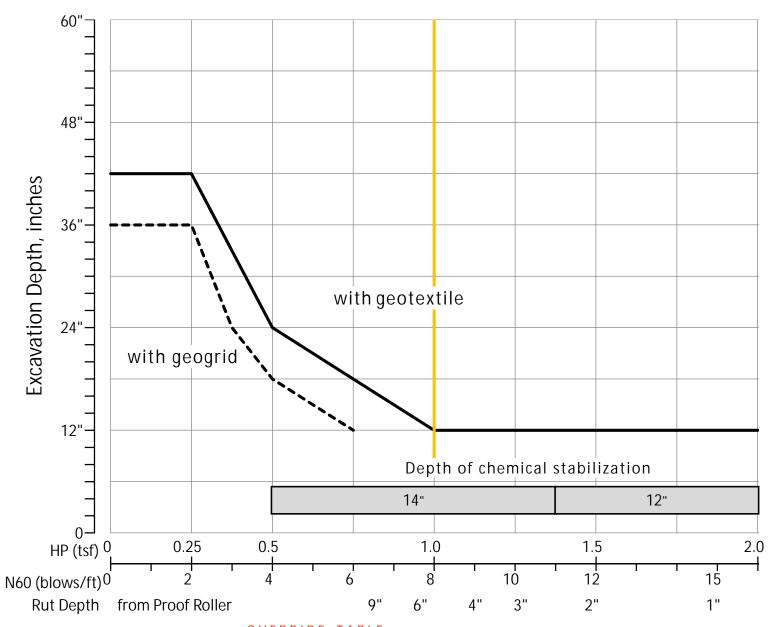
	N <sub>60</sub>	N <sub>60L</sub>	HP	LL	PL	PI	Silt	Clay	P 200	$M_{\mathtt{C}}$	$M_{OPT}$	GI
Average	19	8	3.05	29	18	11	27	37	64	15	12	7
Maximum	100	18	4.50	38	22	17	45	68	89	42	17	16
Minimum	2	2	1.25	19	15	2	6	2	8	5	0	0

					Class	ificat	ion C	ount	s by	Sam	ple								
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	4	0	6	3	0	0	0	7	4	18	2	0	44	11	0	0	0	0	99
Percent	4%	0%	6%	3%	0%	0%	0%	7%	4%	18%	2%	0%	44%	11%	0%	0%	0%	0%	100%
% Rock Granular Cohesive	4%					38%								58	3%				100%
Surface Class Count	1	0	5	2	0	0	0	3	2	15	0	0	30	5	0	0	0	0	63
Surface Class Percent	2%	0%	8%	3%	0%	0%	0%	5%	3%	24%	0%	0%	48%	8%	0%	0%	0%	0%	100%





### GB1 Figure B – Subgrade Stabilization



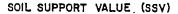
#### OVERRIDE TABLE

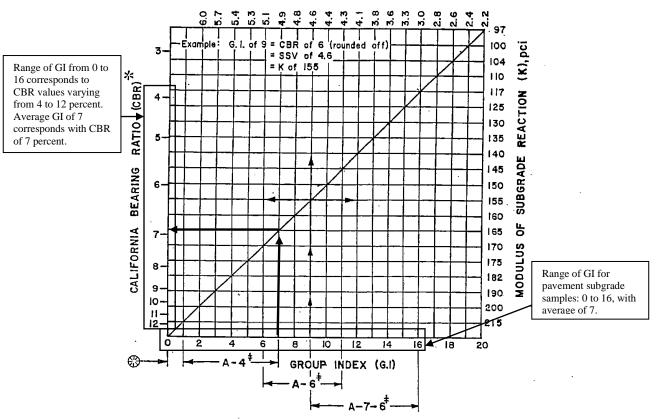
Calculated Average	New Values	Check to Override
3.05		HP
8.09		N60L

Average HP Average N<sub>60L</sub>

#### GEA/LAK-44-18.45/00.00 PID No. 114163

Fig.1301-3 Feb.1978





- † Usual range of AASHTO Classes.
- ☆ 5-1/2 Lb. hammer, 12" drop, 4 layers, 45 blows per layer, compacted
  at optimum moisture as determined by AASHTO T-99.

# CORRELATION CHART FOR SUBGRADE STRENGTHS

The ODOT "Subgrade Analysis" worksheet for the borings at intersections where widening is planned resulted in a CBR value of 7 percent. It should be noted that the CBR determination by the GB-1 spreadsheet is based on an average Group Index (GI) of all the evaluated samples. It should be noted that group indices for the tested samples ranging from 9 to 16, which would correlate with lower CBR values than that determined based on the average GI value of 7, were generally encountered in borings that were indicated by the Subgrade Analysis spreadsheet for subgrade modification, or at depths of at least 1½ feet below the anticipated top of subgrade elevation. As such, it does not appear to be unconservative to use the Subgrade Analysis worksheet design CBR value of 7 percent based on the average design value calculations.

Subgrade modification may consist of undercutting and replacement with new granular engineered fill. Additionally, the Subgrade Analysis worksheet indicates global chemical stabilization to a depth of 14 inches using cement is an option. With widening planned for widths generally 12 feet or less, it is not expected that global chemical stabilization equipment could readily access and treat the subgrades. As such, it is planned that subgrade modification for this project include over-excavation and replacement with new granular engineered fill.



### **APPENDIX B**

(Non-Intersection Logs of Test Borings, Grain Size Distribution, and Photographic Logs of Selected Pavement Cores)



PROJECT: GEA/LAK-44-18.45/00.00  TYPE: ROADWAY  PID: 114163 SFN: N/A  START: 12/12/22 END: 12/12/22	DRILLING FIRM / O SAMPLING FIRM / I DRILLING METHOD SAMPLING METHOD	LOGGER: ):	TT			HAM CALI	MER: BRATI	CME 7: CME A ON DATE: ATIO (%):	UTOM 3/	MATIC	:	STATALIG	NME ATIO	NT: ON:1 <u>2</u>	261.7	(NA\	SR4 /D88)	EOB:		B-001	ATION ID -0-22 PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	N	ELEV. 1261.7	DEPTI	HS	SPT/ RQD	N <sub>60</sub>	REC (%)	SAMPLE ID	HP (tsf)	GR	CS	ATIO FS	N (% sı	) CL	ATT LL	ERBE PL	FI PI	wc	ODOT CLASS (G	SO4 ppm	
ASPHALT - 6 INCHES  CONCRETE - 8 INCHES  VERY STIFF, BROWN, SANDY SILT, LITTL TRACE GRAVEL, DAMP	E CLAY,	1261.2		_ 1 -	6 6 11	19	100	SS-1A	-	6	6	30	40	18	19	13	6	11	A-4a (5	210	
SANDSTONE, BROWN, SEVERELY TO HIC WEATHERED.	GHLY	1258.9	—TR——	_ 3 _	50/5"	-	100	SS-1B SS-2	-	-	-	-	-	-	-	-	-	7	Rock (V Rock (V		

TANDARD ODOT LOG W/ SULFATES (8.5 X 11) - OH DOT.GDT - 7/28

NOTES: AUGER REFUSAL AT 4.3 FT.

PID: 114163 SFN: N/A DRILL	PLING FIRM / LOGGER: _ .LING METHOD:	TTL / KKC 3.5" SSA	•	MER: CME	AUTOM	OK 844 IATIC 15/21	ALIGNM ELEVAT	ENT:		SF	+13, 10' R44 8)EOB:			PAGE
START: <u>12/12/22</u> END: <u>12/12/22</u> SAMF	PLING METHOD:	SPT	ENER	GY RATIO (%)	:	66	COORD		70593	34.7480	N, 2316	032.4090 E	_ 1	1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 1260.0	DEPTHS SPT/RQD	N	REC SAMPLE		GRAI GR CS	DATION (%	6) CL	ATT	ERBER(	_	ODOT CLASS (GI)	SO4 ppm	BACK FILL
ASPHALT - 6 INCHES  CONCRETE - 12 INCHES  DENSE, BROWN, COARSE AND FINE SAND, LITT SILT, TRACE SANDSTONE FRAGMENTS, TRACE DAMP	1259.5 1258.5	- 1 - - 2 - 11 11	32	SS-1A	-	3 19	65 11	2	NP	NP NI	P 5	A-3a (0)	<100	
<b>SANDSTONE</b> , ORANGE, SEVERELY TO HIGHLY WEATHERED.	1207.0	-TR - 18 - 18 - 15 - 50/5"		SS-1B 100 SS-2	-			-	-		8	Rock (V)		

STANDARD ODOT LOG W/ SULFATES (8.5 X 11) - OH DOT.GDT - 7/28/24 15:45 -

NOTES: AUGER REFUSAL AT 4.3 FT.

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 36+03, 10' RT. B-003-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1258.6 (NAVD88)EOB: 1 OF 1 START: 12/12/22 END: COORD: 706314.1680 N, 2315940.0620 E 12/12/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** ELEV. SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1258.6 ASPHALT - 9 INCHES 1257.8 AGGREGATE BASE - 5 INCHES 1 · 1257.4 W Span LOOSE, GRAY/BROWN, COARSE AND FINE SAND, SOME SILT, TRACE GRAVEL, TRACE CLAY, MOIST 2 100 SS-1 33 36 25 2 NP NP NP 11 A-3a (0) <100 3 **w** 1255.1 SS-2A A-3a (V) 100 3 9 1254.5 4 STIFF, GRAY, SILT AND CLAY, TRACE SAND, TRACE 2 3 5 67 34 SS-2B 1.50 23 20 14 20 A-6a (10) GRAVEL, MOIST attalle. SS-3A A-6a (V) 1253.5 5 1000 MEDIUM DENSE, BROWN, FINE SAND, TRACE SILT 20 100 SS-3B A-3 (V) 1253.1 7 > PZ VERY STIFF, BROWN/GRAY, SILT AND CLAY, SOME 1 L SS-3C 3.00 15 A-6a (V) 1252.7 SAND. TRACE GRAVEL. DAMP 49A 6 1252.4 SANDSTONE, BROWN, SEVERELY TO HIGHLY WEATHERED. (FREE WATER NOTED) NOTES: NONE

PROJECT: _ YPE:							AMPLING FIRM / LOGGER: TTL / KKC H					HAMMER: CME AUTOMATIC						STATION / OFFSET: 40+00, 12' L ALIGNMENT: SR44						
PID: 11416		N/A	DRILLING N			3.5" 9			CALI	BRATI	ON DATE:	3/	15/21				_				EOB:			
START:12	<u>MATERIAI</u>	12/12/22 DESCRIPTION	SAMPLING	METHO	DD:	SF		SPT/	ENEI		ATIO (%): SAMPLE		66		COC				88.26 ERBI		, 2315 I	805.1270 E одот	S04	
		NOTES	•		1256.5	DEP1	THS	RQD	N <sub>60</sub>	(%)	ID	(tsf)			FS	$\overline{}$	CL	LL	PL	PI	wc	CLASS (GI)	ppm	
ASPHALT -	7.5 INCHES				1255.9		ļ																	
	E BASE - 6.25 IN				1255.4		_ 1 -																	
	ENSE, BROWN, <b>C</b> , TRACE GRAVEL																							
					1253.9		_ 2 -	5 7 9	18	100	SS-1A	-	4	40	32	22	2	NP	NP	NP	8	A-3a (0)	<100	
	NSE, BROWN, G			0.C			_ 3 -				SS-1B	-	-	1	-	-	-	-	-	-	-	A-1-b (V)	-	
DAMP	,	,	,				-	6 _	44	400	00.0		0	47	0.5	40	0	ND	ND	NE		A 4 L (0)		
							_ 4 -	5 5	11	100	SS-2	-	3	47	35	13	2	NP	NP	NP	4	A-1-b (0)	-	
\( (= \) \( (= \) \( (= \) \( (= \) \( (= \) \)			0.107		1251.5		- 5 -	3			SS-3A	-	-	-	-	-	-	-	-	-	-	A-1-b (V)	-	
LITTLE GRA	F, BROWN, <b>SAND</b> AVEL, TRACE SHA	ALE FRAGMEN	ITS, DAMP		1250.9		-	7	19	100	SS-3B	-	17	10	10	28	35	25	15	10	11	A-4a (6)	-	
	WN, <b>SANDY SIL</b> Y, TRACE SAND						- 6 -				SS-3C	-	-	-	-	-	-	-	-	-	-	A-4a (V)	-	
					1249.2		- - 7 -	12 20 30	55	89	SS-4A	-	•	1	-	-	-	-	-	-	12	A-4a (V)	-	
SANDSTON	E, SEVERELY TO		TUEDED		1249.0	——TR— <b>—</b> ЕОВ <b>—</b>	-  I				SS-4B										-	Rock (V)	<u> </u>	

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 43+88, 10' RT. B-005-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: **PAGE** PID: \_\_114163\_\_ SFN: \_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1252.7 (NAVD88)EOB: 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 707065.7840 N, 2315715.3690 E 12/12/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1252.7 ASPHALT - 6.5 INCHES 1252.2 **CONCRETE - 11.5 INCHES** - 1 -1251.2 HARD, GRAY, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, DAMP 2 100 SS-1 4.50 7 8 22 54 28 17 11 A-6a (8) 270 1249.7 3 VERY STIFF, GRAY/BROWN, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, MOIST 18 100 SS-2 2.75 4 6 11 23 56 31 | 17 14 17 A-6a (10) THE PARTY OF THE P J 050 SS-3A A-6a (V) 5 > 200 22 100 10 1247.3 2 L VERY STIFF, GRAY, SILTY CLAY, LITTLE SAND, 400 SS-3B 3.50 18 A-6b (V) TRACE GRAVEL, MOIST 6 WBM2 8 18 100 SS-4 3.75 18 A-6b (V) 1245.2 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 47+89, 10' LT. B-006-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1246.4 (NAVD88)EOB: 7.5 ft. 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 707444.6400 N, 2315581.5890 E 12/12/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1246.4 ASPHALT - 7 INCHES 1245.8 **CONCRETE - 11 INCHES** - 1 -1244.9 STIFF, BROWN, SANDY SILT, LITTLE CLAY, TRACE GRAVEL, DAMP SS-1A 3 38 42 10 18 14 4 11 A-4a (3) 2 13 100 @2.3': GRAY/BROWN, TRACE CLAY 7 SS-1B 13 31 45 23 15 8 10 A-4a (3) 210 3 1243.2 SS-2A A-4a (V) VERY STIFF, BROWN, SILT AND CLAY, AND SAND. LITTLE GRAVEL, DAMP 15 100 SS-2B 25 26 33 3.75 11 | 14 24 19 14 17 A-6a (5) 4 400p N OSD 5 22 SS-3 A-6a (V) 400 6 @6': TRACE GRAVEL (VS) 6 17 100 SS-4 3.75 15 A-6a (V) 1 2 2 A L 1238.9 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 51+83, 5' RT. B-007-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1240.2 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: COORD: 707826.6720 N, 2315483.4650 E 12/12/22 SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1240.2 ASPHALT - 7.5 INCHES 1239.6 BROWN, COARSE AND FINE SAND, SOME SILT - 1 -1238.9 HARD, BROWN/GRAY, **SANDY SILT**, SOME CLAY, TRACE GRAVEL, DAMP 2 14 100 SS-1 4.50 3 8 18 43 28 26 16 10 12 A-4a (7) 260 @2.2': 1-INCH SAND SEAM 3 SS-2A A-4a (V) 1236.7 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, 7 15 100 LITTLE GRAVEL. DAMP SS-2B 2.75 10 26 46 29 16 4 4 14 13 15 A-6a (9) THE STATE OF THE S N OSD 5 18 100 SS-3 A-6a (V) 16 2 L 400 6 (VS) @6.3': GRAY/BROWN. LITTLE SAND. LITTLE SANDSTONE FRAGMENTS 22 100 SS-4 10 13 A-6a (V) 1 2 2 10 A L 1232.7 NOTES: NONE

ROJECT: GEA/LAK-44-18.45/00.00 PE: ROADWAY D: 114163 SFN: N/A	DRILLING FIRM / OPERATOR: TTL / CW SAMPLING FIRM / LOGGER: TTL / KKC DRILLING METHOD: 3.5" SSA						DRILL RIG: CME 75 TRUCK 844 HAMMER: CME AUTOMATIC CALIBRATION DATE: 3/15/21 ENERGY PATIO (%): 66					ALIC	NME VATIO	NT: ON:1 <u>2</u>	233.9					EXPLORATIO B-008-0-22 .5 ft. PA		
ART: <u>12/12/22</u> END: <u>12/12/22</u> <i>MATERIAL DESCRIPTIO</i>				SPT SPT/		N <sub>60</sub>	RGY RATIO (%):		HP			DATION (%)			708191.7280 N ATTERBERG				ODOT	SO4	В	
AND NOTES SPHALT - 10.5 INCHES	, , ,		1233.9	J		RQD	• •60	(%)	<u>ID</u>	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	I W
GGREGATE BASE - 6.5 INCHES	, , ,		1233.0		— 1 –																	×××××××××××××××××××××××××××××××××××××
OOSE, BROWN, <b>COARSE AND FINE SAN</b> ILT, TRACE GRAVEL, TRACE CLAY, DAM			1231.4	-	_ 2 -	3 4 4	9	89	SS-1A	-	6	32	39	21	2	NP	NP	NP	9	A-3a (0)	-	- 78 L V & W
STIFF, GRAY, <b>SANDY SILT</b> , SOME CLAY, TRACE GRAVEL, DAMP					- - 3 -				SS-1B	-	4	11	20	41	24	24	15	9	13	A-4a (6)	-	7
				_ 4 -	4 5 7	13	100	SS-2A	-	7	10	16	44	23	24	15	9	12	A-4a (6)	-	2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
TIFE ODAY OH TANDOLAY COME CAN	DUTTIE		1229.4	-	_				SS-2B	-	-	-	-	-	-	-	-	-	-	A-4a (V)	150	-3
TIFF, GRAY, <b>SILT AND CLAY</b> , SOME SAN HALE FRAGMENTS, DAMP	D, LITTLE		1227.9		5	7 5 9	15	100	SS-3	-	-	-	-	-	-	-	-	-	12	A-6a (V)	-	DITTO TO THE PERSON THE PERSON TO THE PERSON TO THE PERSON TO THE PERSON TO THE PERSON
ARD, GRAY, <b>SILT AND CLAY</b> , SOME SAN HALE FRAGMENTS, DAMP	D, LITTLE			_	6 7	7 9 11	22	100	SS-4	4.50	-	-	-	-	-	-	-	-	12	A-6a (V)	-	\$ VA & & & A & A & A & A
NALE FRAGINENTS, DAINF			1226.4	EOB—	- - 7 -		22	100	SS-4	4.50	-	-	-	-	-	-	-	-	12	A-6a (V)		-

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 59+63, 9' RT. B-009-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1227.8 (NAVD88)EOB: 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: COORD: 708575.4840 N, 2315263.7910 E 12/12/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1227.8 ASPHALT - 6 INCHES 1227.3 **CONCRETE - 12 INCHES** - 1 -1226.3 **w** 1226.3 1 2 2 LOOSE, BROWN, FINE SAND, TRACE SILT, MOIST SS-1A 13 A-3 (V) 1225.8 2 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, 2 6 100 TRACE GRAVEL, DAMP 46 17 SS-1B 3.00 5 9 14 26 28 11 16 A-6a (8) 250 3 @3': MOIST 2 47 A-6a (10) 100 SS-2 2.00 2 7 17 27 34 19 15 21 CHARAS J 050 5 SS-3A A-6a (V) Z > EZ 10 100 1 L 1222.3 STIFF, BROWN, SILTY CLAY, LITTLE SAND, TRACE SS-3B 2.00 19 A-6b (V) SOM GRAVEL, TRACE SANDSTONE FRAGMENTS, MOIST 6 WBM2 7 > L @6.5': GRAY 5 100 SS-4 11 1.00 A-6b (V) 1220.3 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 63+54, 13' LT. B-010-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: **PAGE** PID: \_\_114163 \_\_ SFN: \_\_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1221.2 (NAVD88)EOB: 1 OF 1 START: 12/14/22 END: 12/14/22 SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 708943.5040 N, 2315131.1720 E SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1221.2 ASPHALT - 6.5 INCHES 1220.7 **CONCRETE - 8 INCHES** - 1 -1220.0 1219.8 BROWN, **FINE SAND**, TRACE SILT VERY STIFF, GRAY/BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 2 3 100 SS-1 3.50 2 7 18 23 50 33 | 18 15 17 A-6a (10) 250 1218.2 3 HARD, BROWN/GRAY, SILT AND CLAY, SOME SAND. TRACE SHALE FRAGMENTS, DAMP 28 100 SS-2 4.50 13 A-6a (V) THE PARTY OF THE P N 050 5 9 > M 15 100 SS-3 7 | 11 | 15 25 42 27 16 11 13 A-6a (7) 2011A 400 6 WBM2 12 33 100 SS-4 14 4.00 13 A-6a (V) 16 1213.7 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 67+39, 9' RT. B-011-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1213.0 (NAVD88)EOB: 7.5 ft. 1 OF 1 START: 12/14/22 END: SAMPLING METHOD: COORD: 709318.3550 N, 2315401.7160 E 12/14/22 SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1213.0 ASPHALT - 9.5 INCHES 1212.2 AGGREGATE BASE - 8.5 INCHES 1 · W Span 1211.5 MEDIUM DENSE, BROWN, FINE SAND, TRACE SILT, 7/2 SS-1A 10 A-3 (V) DAMP 1210.9 2 HARD, BROWN, SILT AND CLAY, SOME SAND, DAMP 14 100 A-6a (9) SS-1B 4.50 0 6 15 28 51 32 19 13 15 250 3 @3': VERY STIFF, TRACE GRAVEL 6 15 100 SS-2 3.25 2 8 14 21 55 30 18 12 15 A-6a (9) 1208.5 attitiko ( HARD. BROWN. SANDY SILT. LITTLE CLAY. TRACE GRAVEL, DAMP 5 N SED 12 28 100 SS-3 4.50 13 A-4a (V) 7 > PZ 1207.0 400 6 HARD. BROWN. SILT AND CLAY. SOME SAND. TRACE GRAVEL, TRACE SHALE FRAGMENTS, DAMP W Span 7/2/20 W/1/20 W/ 41/20 20 55 100 SS-4 25 13 A-6a (V) 25 1 2 THE PARTY OF THE P 1205.5 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

X:\PROJECTS\2279801.GPJ

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 71+22, 14' LT. B-012-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1201.9 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/14/22 END: COORD: 12/14/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 709679.2370 N, 2314909.7350 E ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1201.9 ASPHALT - 10 INCHES 1201.1 AGGREGATE BASE - 5.5 INCHES 1 · 1200.6 WSAN MEDIUM DENSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, DAMP NP SS-1A 47 30 12 2 NP NP 2 9 A-1-b (0) 15 100 SS-1B <100 A-1-b (V) 1198.9 3 LOOSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, DAMP 2 NP NP NP SS-2A 27 37 26 8 A-1-b (0) 100 3 8 1197.6 VERY STIFF. BROWN. SANDY SILT. SOME CLAY. SS-2B A-4a (V) attitiko. TRACE GRAVEL, DAMP 5 N 900 22 100 SS-3 9 10 14 38 29 26 17 9 14 A-4a (6) 7 > PZ 400 6 @6': HARD 480m 7 > L The second of the second 35 100 SS-4 15 4.50 A-4a (V) 17 1 2 1194.4 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

GPJ.

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 75+10, 5' RT. B-013-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1190.5 (NAVD88)EOB: PID: \_\_114163\_\_ SFN: \_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/14/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 12/14/22 SPT 66 710056.1780 N, 2314816.7230 E **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1190.5 ASPHALT - 6.5 INCHES 1190.0 **CONCRETE - 8.5 INCHES** 1189.2 **w** 1189.2 AGGREGATE BASE - 5 INCHES 1188.8 SS-1A A-1-b (V) STIFF, GRAY, SANDY SILT, AND CLAY, TRACE Z L GRAVEL, DAMP 2 12 100 A-4a (8) SS-1B 6 10 38 39 24 16 8 14 250 1187.5 3 STIFF, BROWN/GRAY, SILT AND CLAY, SOME SAND. TRACE GRAVEL, DAMP 5 13 100 SS-2 5 7 17 25 46 31 17 14 16 A-6a (9) GRAP. @4.5': VERY STIFF, BROWN, MOIST N 050 5 25 100 SS-3 3.25 18 A-6a (V) 11 2 L 400 6 WBM2 @6': HARD, GRAY, DAMP 9 100 SS-4 19 4.50 16 A-6a (V) 1183.0 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 98+47, 12' LT. B-019-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1123.0 (NAVD88)EOB: PID: \_\_114163 \_\_ SFN: \_\_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/15/22 END: 12/15/22 SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 712298.3990 N, 2314156.1910 E SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1123.0 ASPHALT - 6.5 INCHES 1122.5 **CONCRETE - 8.5 INCHES** - 1 -1121.7 BROWN, FINE SAND, TRACE SILT 1121.3 SS-1A A-3 (V) STIFF, BROWN, SILT AND CLAY, SOME SAND, Z LXD TRACE GRAVEL, DAMP 2 3 100 SS-1B 6 23 24 38 28 | 17 11 15 A-6a (6) 260 3 39 18 45 100 SS-2 2 11 23 25 27 | 15 12 13 A-6a (7) @3.8': HARD 4 GAZA. 1118.5 VERY STIFF, GRAY, SILTY CLAY, LITTLE SAND, D 0500 TRACE GRAVEL, DAMP 5 18 > 200 19 100 SS-3 3.25 A-6b (V) 14 2011A 400° 6 WBM2 @6': TRACE SHALE FRAGMENTS 35 100 SS-4 13 3.00 A-6b (V) 19 1115.5 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 102+37, 9' RT. B-020-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: \_\_114163\_\_ SFN: \_\_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1120.4 (NAVD88)EOB: 1 OF 1 START: 12/15/22 END: 12/15/22 SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 712682742.0000 N, 2314090.7470 E SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1120.4 ASPHALT - 8.5 INCHES 1119.7 AGGREGATE BASE - 8.5 INCHES - 1 -1119.0 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 2 3 100 SS-1 4.00 3 7 17 26 47 30 17 13 16 A-6a (9) 270 3 @3.5': TRACE SHALE FRAGMENTS 47 8 20 100 SS-2 2 9 14 28 30 | 17 13 14 A-6a (9) 1115.9 VERY STIFF, GRAY/BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, TRACE ORGANICS, MOIST N 990 5 32 100 SS-3 3.75 17 A-6a (V) 13 7 > M 400 6 WBAN @6.3': BROWN 15 ا ح لا 30 100 SS-4 15 17 A-6a (V) 12 A L 1112.9 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 106+30, 15' LT. B-021-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1121.6 (NAVD88)EOB: 1 OF 1 START: 12/15/22 END: SAMPLING METHOD: COORD: 713063.0370 N, 2313988.3420 E 12/15/22 SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1121.6 ASPHALT - 9 INCHES 1120.8 AGGREGATE BASE - 5.5 INCHES - 1 -1120.4 VERY STIFF, BROWN/GRAY, SILTY CLAY, SOME SAND, TRACE GRAVEL, MOIST 2 12 100 SS-1 2.50 5 16 25 53 35 18 17 A-6b (11) 270 1 @2.8': BROWN, TRACE SANDSTONE FRAGMENTS 3 SS-2A 2.50 19 A-6b (V) 1117.8 6 13 100 STIFF, BROWN, SILT AND CLAY, SOME SAND, SOME SANDSTONE FRAGMENTS, TRACE GRAVEL, DAMP SS-2B A-6a (V) attitiko ( 5 N SED 15 100 SS-3 23 10 20 20 27 29 18 11 11 A-6a (3) 7 > PZ 400 6 @6': VERY STIFF W Spin 12 12 26 100 SS-4 15 A-6a (V) 12 1 2 1114.1 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 110+20, 9' RT. B-022-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1123.3 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/15/22 END: SAMPLING METHOD: COORD: 713451.6930 N, 2313943.1280 E 12/15/22 SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1123.3 ASPHALT - 6.5 INCHES 1122.8 CONCRETE - 8.5 INCHES - 1 -1122.0 BROWN, FINE SAND, TRACE SILT 1121.6 SS-1A A-3 (V) MEDIUM STIFF, BROWN, SILT AND CLAY, SOME Z LXD SAND, TRACE GRAVEL, DAMP 2 8 100 A-6a (8) SS-1B 9 8 13 26 44 30 18 12 15 270 3 SS-2A A-6a (V) 1120.0 STIFF. BROWN. SILT AND CLAY. SOME SAND. TRACE GRAVEL, MOIST 7 15 100 SS-2B 3 11 22 25 39 29 17 12 17 A-6a (7) 4 THE P @4.5': VERY STIFF, TRACE SHALE FRAGMENTS, D 0500 DAMP 5 > 200 21 100 SS-3 3.75 15 A-6a (V) 2011A 400 6 WBM2 @6': LITTLE GRAVEL 10 33 100 SS-4 15 13 A-6a (V) 15 1115.8

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 114+18, 13' LT. B-023-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE PID: \_\_114163\_\_ SFN: \_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1124.8 (NAVD88)EOB: 1 OF 1 START: 12/15/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 713841.5190 N, 2313859.3020 E 12/15/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1124.8 ASPHALT - 6 INCHES 1124.3 **CONCRETE - 8.75 INCHES** - 1 -1123.5 BROWN, FINE SAND, TRACE SILT 1123.1 SS-1A A-3 (V) STIFF, GRAY, SANDY SILT, AND CLAY, TRACE Z L GRAVEL, DAMP 2 13 100 SS-1B 5 8 10 38 39 26 17 9 13 A-4a (8) 270 1121.8 3 VERY STIFF, GRAY, SILT AND CLAY, SOME SAND. TRACE GRAVEL, DAMP - 7/28/24 15:46 - X:\PROJECTS\2279801.GPJ 5 12 100 SS-2 2.75 15 A-6a (V) THE PARTY OF THE P NOD 5 14 100 SS-3 2.50 4 9 17 27 43 27 16 11 15 A-6a (7) 2011A 400 6 WBM2 100 SS-4 2.50 10 21 15 A-6a (V) 1117.3 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 118+06, 9' RT. B-024-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1124.4 (NAVD88)EOB: 1 OF 1 START: 12/15/22 END: COORD: 714227.8300 N, 2313821.2790 E 12/15/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1124.4 ASPHALT - 9 INCHES 1123.6 AGGREGATE BASE - 6 INCHES 1 · 1123.1 MEDIUM DENSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, DAMP 2 14 100 SS-1 12 44 37 5 2 NP NP NP A-1-b (0) <100 1121.4 3 VERY LOOSE. BROWN. COARSE AND FINE SAND. SOME SILT, TRACE GRAVEL, TRACE CLAY, DAMP NP NP 2 100 SS-2 5 44 25 22 4 NP A-3a (0) attitiko ( SS-3A A-3a (V) 5 N 900 1119.1 100 VERY LOOSE, BROWN, GRAVEL AND STONE 7 > PZ FRAGMENTS, TRACE SAND, TRACE SILT, DRY NP SS-3B 91 4 4 NP NP A-1-a (0) 400 1118.3 6 STIFF, GRAY, SILT AND CLAY, LITTLE SAND, LITTLE WSDD GRAVEL. LITTLE SHALE FRAGMENTS. DAMP 100 SS-4 10 12 A-6a (V) احيلا 1 2 1116.9 CONTRACT. NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 121+93, 13' LT. B-025-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1123.0 (NAVD88)EOB: 7.5 ft. 1 OF 1 START: 12/15/22 END: SAMPLING METHOD: COORD: 714606.3790 N, 2313739.5380 E 12/15/22 SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1123.0 ASPHALT - 9 INCHES 1122.2 AGGREGATE BASE - 6 INCHES - 1 -W Span 1121.7 LOOSE, BROWN, COARSE AND FINE SAND, SOME SILT, TRACE GRAVEL, TRACE CLAY, DAMP 2 3 8 100 SS-1 A-3a (V) 3 @3': VERY LOOSE 2 NP NP A-3a (0) 100 SS-2 5 44 25 22 4 NP <100 THE STATE OF THE S SS-3A A-3a (V) 1117.9 5 1000 MEDIUM STIFF, BROWN, SANDY SILT, SOME 6 100 GRAVEL, LITTLE CLAY, DAMP M> > D SS-3B 23 12 20 35 10 18 16 2 11 A-4a (2) 49A 6 SS-4A A-4a (V) WSDD 1116.5 7 > L WILL X & AL **SANDSTONE. BROWN. SEVERELY TO HIGHLY** 20 47 100 WEATHERED, TRACE SHALE FRAGMENTS. 23 1 2 SS-4B Rock (V) THE PARTY OF THE P 1115.5 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 125+82, 9' RT. B-026-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: \_\_114163\_\_ SFN: \_ N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION:1121.4 (NAVD88)EOB: 1 OF 1 START: 12/15/22 END: COORD: 714994.6890 N, 2313701.5280 E 12/15/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG **MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1121.4 ASPHALT - 6.5 INCHES 1120.9 **CONCRETE - 9 INCHES** - 1 -1120.1 AGGREGATE BASE - 6 INCHES SS-1A A-1-b (V) 1119.6 HARD, GRAY, SILT AND CLAY, SOME SAND, TRACE 2 GRAVEL. DAMP 3 100 SS-1B 4.50 1 6 14 21 58 31 18 13 16 A-6a (9) 250 1118.4 3 VERY STIFF, BROWN/GRAY, SILT AND CLAY, LITTLE SAND, MOIST 5 14 100 SS-2 3.25 0 4 12 25 59 33 | 19 14 20 A-6a (10) THE PARTY OF THE P @4.5': HARD N 050 5 24 100 SS-3 4.25 A-6a (V) 10 24 2 L 400 6 WBM2 @6': VERY STIFF, GRAY/BROWN, SOME SAND, TRACE **GRAVEL** 10 29 100 SS-4 12 3.25 19 A-6a (V) 14 1113.9 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 129+71, 14' LT. B-027-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1119.9 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/15/22 END: COORD: 12/15/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 715375.4080 N, 2313618.4510 E ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1119.9 ASPHALT - 6.5 INCHES 1119.4 **CONCRETE - 10.5 INCHES** - 1 -1118.5 MEDIUM DENSE. BROWN. GRAVEL AND STONE FRAGMENTS WITH SAND AND SILT, TRACE BRICK NP A L SS-1A 45 20 8 24 3 NP NP A-2-4 (0) <100 FRAGMENTS. TRACE CLAY, DAMP FILL 2 10 1117.6 26 100 DENSE, BROWN, **SANDY SILT**, SOME GRAVEL, TRACE CLAY, DAMP SS-1B A-4a (V) 3 NP NP NP 12 33 100 SS-2 33 | 11 16 31 9 A-4a (1) 4 CHARAS 1115.4 VERY DENSE, BROWN, FINE SAND, LITTLE GRAVEL. D 0500 TRACE SHALE FRAGMENTS, TRACE SILT, DAMP 5 18 ₩>₩ 56 100 SS-3 5 A-3 (V) 2011A 400 6 WBM2 12 \_ 27 100 SS-4 56 A-3 (V) 24 1112.4 -EOB-NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 152+93, 13' LT. B-033-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1094.5 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: SAMPLING METHOD: COORD: 717669.4060 N, 2313261.6610 E 12/19/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1094.5 ASPHALT - 9 INCHES 1093.7 AGGREGATE BASE - 6 INCHES 1 · 1093.2 WSAN MEDIUM DENSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, DAMP 2 14 100 SS-1 14 40 35 8 3 NP NP NP A-1-b (0) <100 1091.5 3 VERY LOOSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, DAMP 2 NP NP 100 SS-2 8 47 36 7 2 NP A-1-b (0) 4 1090.0 THE P LOOSE, BROWN, COARSE AND FINE SAND, SOME CLAY, LITTLE SILT, DAMP **w** 1089.5 5 N SED 6 89 SS-3 A-3a (V) 7 > PZ 4 6 @6': MEDIUM DENSE SS-4A 5 A-3a (V) Wann 1088.0 13 DENSE, GRAY, FINE SAND, TRACE SILT, MOIST 45 100 18 ا ح آلا 23 1 2 SS-4B 18 A-3 (V) THE PARTY OF THE P 1087.0 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 156+85, 10' RT. B-034-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1088.7 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/19/22 END: COORD: 718060.9730 N, 2313223.3720 E 12/19/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1088.7 ASPHALT - 9 INCHES 1087.9 AGGREGATE BASE - 6 INCHES 1 · 1087.4 WSAN LOOSE, BROWN, GRAVEL AND/OR STONE FRAGMENTS, LITTLE SILT, TRACE CLAY, DAMP (CRUSHED STONE) FILL 2 10 100 SS-1 15 37 27 18 3 NP NP NP A-1-b (0) <100 3 @3': TRACE SILT NP NP 3 7 100 SS-2 4 52 34 8 2 NP A-1-b (0) 4 attitiko ( **w** 1083.9 @4.8': MEDIUM DENSE, LITTLE SANDSTONE 5 N 900 FRAGMENTS, TRACE BRICK FRAGMENTS SS-3A A-1-b (V) 13 100 1082.9 -TR-4 SS-3B Rock (V) SANDSTONE. BROWN. SEVERELY TO HIGHLY 6 WEATHERED. WSDD 7 > L The second of the second 6 15 100 SS-4 13 Rock (V) 1 2 1081.2 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 160+72, 10' LT. B-035-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1083.0 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 12/19/22 SPT 66 718439.4150 N, 2313143.6000 E **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1083.0 ASPHALT - 6.5 INCHES 1082.5 **CONCRETE - 8.25 INCHES** 1 · 1081.8 BROWN, FINE SAND, TRACE SILT 1081.4 SS-1A - -A-3 (V) STIFF, BROWN, SANDY SILT, LITTLE CLAY, TRACE GRAVEL. DAMP 2 SS-1B 9 14 23 37 17 25 15 10 12 A-4a (4) 13 100 1080.5 VERY STIFF, GRAY/BROWN, SILTY CLAY, SOME SS-1C A-6b (V) 130 SAND. TRACE GRAVEL. MOIST 3 42 11 100 SS-2 2.50 1 8 21 28 35 19 16 20 A-6b (9) 260 THE PARTY OF THE P J 050 5 > 200 22 100 SS-3 2.50 19 A-6b (V) 10 2011A 400 6 WBM2 @6': STIFF TO VERY STIFF SS-4A A-6b (V) 1.75 19 @6.5': BROWN, LITTLE SANDSTONE FRAGMENTS, 7 7 22 100 10 DAMP 10 SS-4B 16 A-6b (V) A Land 1075.5 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 164+61, 10' RT. B-036-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1077.1 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/19/22 END: SAMPLING METHOD: COORD: 718827.3350 N, 2313104.1240 E 12/19/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1077.1 ASPHALT - 7 INCHES 1076.5 **CONCRETE - 8 INCHES** - 1 -1075.8 LOOSE, BROWN, FINE SAND, TRACE SILT, DAMP 1 2 2 2 1075.4 SS-1A A-3 (V) MEDIUM STIFF, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 2 3 100 SS-1B 6 12 18 26 38 28 17 11 14 A-6a (6) 260 1074.1 3 STIFF, BROWN, SILT AND CLAY, SOME SAND. TRACE GRAVEL, DAMP 5 12 100 SS-2 6 9 18 | 24 43 31 | 18 13 17 A-6a (8) THE STATE OF THE S @4.5': VERY STIFF, GRAY/BROWN N OSD 5 10 22 100 SS-3 3.75 17 A-6a (V) 2 L 400 1071.1 6 VERY STIFF, BROWN, SILTY CLAY, TRACE SAND. (VS) MOIST 10 12 26 100 SS-4 3.00 A-6b (V) 1 2 2 12 A L 1069.6 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

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EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 168+42, 10' LT. B-037-0-22 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 **PAGE** ELEVATION:1071.6 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/12/22 END: COORD: 719200.5240 N, 2313025.1660 E 12/12/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1071.6 ASPHALT - 9 INCHES 1070.8 AGGREGATE BASE - 6 INCHES - 1 -W Spin 1070.3 MEDIUM DENSE, BROWN, FINE SAND, LITTLE 7/21 SANDSTONE FRAGMENTS, TRACE SILT, DAMP 1 / N SS-1A 9 A-3 (V) 1069.6 2 STIFF, BROWN/GRAY, SILT AND CLAY, SOME SAND, 5 11 100 TRACE SANDSTONE FRAGMENTS, MOIST SS-1B 25 1 6 18 50 31 17 14 17 A-6a (10) 250 1068.6 3 VERY STIFF, GRAY, SILT AND CLAY, SOME SAND. TRACE GRAVEL, MOIST 5 12 100 SS-2 2.75 1 7 18 26 48 31 | 18 13 19 A-6a (9) 4 attitiko ( @4.5': HARD. DAMP 5 N 900 20 100 SS-3 4.50 15 A-6a (V) 7 > PT 400 6 @6': VERY STIFF, MOIST WSDD 4 > C 9 22 100 SS-4 3.25 A-6a (V) 11 1 2 1064.1 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 172+25, 11' RT. B-038-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1068.2 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 719581.7600 N, 2312986.5210 E 12/12/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1068.2 ASPHALT - 4.5 INCHES 1067.8 **CONCRETE - 9 INCHES** 1067.1 - 1 -BROWN, FINE SAND, TRACE SILT 1066.7 VERY STIFF, BROWN, SILT AND CLAY, AND SAND, TRACE GRAVEL, MOIST 2 100 SS-1 3.50 2 12 23 26 37 32 17 15 17 A-6a (8) 250 1065.2 3 VERY STIFF, BROWN, SILTY CLAY, SOME SAND. TRACE GRAVEL, DAMP 5 13 100 SS-2 3.75 5 8 17 24 46 37 19 18 18 A-6b (10) ARIA DO @4.5': HARD N 950 5 Ø > ₽ 28 100 SS-3 4.50 16 A-6b (V) 1 L 11 Agod . 1062.2 (FEAN) 6 VERY STIFF, GRAY/BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 34 100 SS-4 18 3.25 17 A-6a (V) 13 1060.7 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

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EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 176+13, 14' LT. B-039-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION:1065.8 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/12/22 END: COORD: 719961.8870 N, 2312902.4740 E 12/12/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI mag (%) CL LL PL PΙ WC **AND NOTES** ID 1065.8 ASPHALT - 7 INCHES 1065.2 **CONCRETE - 8 INCHES** 1 · 1064.5 BROWN, FINE SAND, TRACE SILT 1064.1 SS-1A A-3 (V) VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP 2 SS-1B 16 23 49 30 17 4 8 13 13 A-6a (9) 17 100 SS-1C A-6a (V) 170 3 @3.5': LITTLE GRAVEL, TRACE SHALE FRAGMENTS 19 100 SS-2 10 6 12 26 46 29 18 11 14 A-6a (8) 250 10 THE STATE OF THE S N OSD SS-3A A-6a (V) 5 13 51 100 27 > POZ 1060.4 1 L VERY DENSE, BROWN, GRAVEL AND STONE SS-3B 12 A-1-b (V) 400 FRAGMENTS WITH SAND, TRACE SILT, TRACE CLAY, MOIST 6 (VS) SS-4A A-1-b (V) 1059.3 17 727 HARD. GRAY. SILT AND CLAY. SOME SAND. LITTLE 47 100 20 1 2 2 GRAVEL, TRACE SANDSTONE FRAGMENTS, DAMP 23 SS-4B 12 A-6a (V) A L 1058.3 NOTES: NONE

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 180+08, 11' RT. B-040-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION:1063.4 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 720355.4770 N, 2312865.9350 E 12/12/22 SPT 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1063.4 ASPHALT - 6 INCHES 1063.0 **CONCRETE - 7.5 INCHES** 1062.3 - 1 -LOOSE, BROWN, FINE SAND, TRACE SILT, MOIST SS-1A 14 A-3 (V) <100 1061.4 2 STIFF, BROWN, SILT AND CLAY, LITTLE SAND, 12 100 TRACE GRAVEL, DAMP SS-1B 2 5 23 57 17 13 29 12 14 A-6a (9) 3 SS-2A A STAN A-6a (V) 1059.7 14 100 STIFF, BROWN, SANDY SILT, LITTLE CLAY, TRACE GRAVEL, DAMP SS-2B 9 9 24 39 19 22 15 7 12 A-4a (5) 250 THE STATE OF THE S N 95/2 SS-3A A-4a (V) 5 7 > PZ 10 1058.1 18 100 2 L VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, 400 TRACE GRAVEL. DAMP SS-3B 16 A-6a (V) (FEAN) 6 @6.1': LITTLE SAND, LITTLE SANDSTONE FRAGMENTS 1 2 2 2 9 22 100 SS-4 16 A-6a (V) 11 1055.9

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 183+96, 10' LT. B-041-0-22 SR44 TYPE: ROADWAY SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: **PAGE** ELEVATION:1061.3 (NAVD88)EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 1 OF 1 START: 12/12/22 END: SAMPLING METHOD: **ENERGY RATIO (%):** COORD: 12/12/22 SPT 66 720736.4310 N, 2132785.5420 E **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%)** ATTERBERG SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 1061.3 ASPHALT - 4 INCHES 1061.0 AGGREGATE BASE - 10 INCHES 1 · 1060.1 MEDIUM DENSE, BROWN, COARSE AND FINE SAND. SOME SILT, TRACE GRAVEL, TRACE CLAY, DAMP 2 15 100 SS-1 8 39 29 22 2 NP NP NP A-3a (0) <100 1058.3 3 VERY LOOSE, BROWN, GRAVEL AND STONE FRAGMENTS WITH SAND, LITTLE SILT, TRACE CLAY, DAMP 2 NP NP NP 100 SS-2 15 | 42 25 16 2 A-1-b (0) THE STATE OF THE S N 000 ₹>₹ 5 6 100 SS-3 A-1-b (V) 400 @5.3': LOOSE WBAN 6 SS-4A A-1-b (V) 1054.8 HARD. BROWN. SILT AND CLAY. LITTLE GRAVEL. 6 13 100 LITTLE SAND, DAMP SS-4B 4.25 16 A-6a (V) 1053.8 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

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	PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERA TYPE: ROADWAY SAMPLING FIRM / LOGG								DRILL RIG: CME 75 TRUCK 844 HAMMER: CME AUTOMATIC					STATION / OFFSET: 234+99, 10' RT. ALIGNMENT: SR44 EXPLORATION B-054-0-22									
PID: <u>114163</u> SFN: <u>N/A</u> DRILLING METHO									CALIBRATION DATE: 3/15/21								989.0 (NAVD88) EOB:7			7.5 ft.	.5 ft. F		
START: <u>12/21</u>	1/22 END: <u>12/21/22</u>	SAMPLING M	AMPLING METHOD: SPT				ENERGY RATIO (%): 66						COORD: 725804.1570 N, 2312729.23					729.2310 E		10			
MATERIAL DESCRIPTION				ELEV.	DEPT	DEPTHS SPT/				SAMPLE	AMPLE HP				DATION (%)		ATTERBERG				ODOT	SO4	
	AND NOTES		NAA	989.0		110	RQD	1 160	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	
	- 4 INCHES BROWN, <b>SILT AND CLAY</b> , LI <sup>T</sup> LE SAND, TRACE ORGANICS			988.7		- - 1 -	3 4 7	12	83	SS-1	-	-	-	-	-	-	-	1	-	20	A-6a (V)	-	N V A K A K A L
@1.5': VERY STIFF  VERY STIFF, GRAY, SILT AND CLAY, LITTLE SAND, TRACE GRAVEL, TRACE ORGANICS, DAMP			986.8		_ 2 _	3	_		SS-2A	2.50	-	-	-	-	-	-	1	-	20	A-6a (V)	-	7 7 7 7	
				986.4		-	3 3	7	89	SS-2B	3.25	3	3	9	25	60	33	22	11	20	A-6a (8)	-	2 / Vall
HARD, BROWN TRACE GRAVE	N, <b>SILT AND CLAY</b> , LITTLE S EL, DAMP	AND,				_ 3 -	_			SS-2C	-	-	-	-	-	-	-	-	-	-	A-6a (V)	-	7
				985.5						SS-3A	4.50	1	5	11	25	58	33	20	13	15	A-6a (9)	240	1
VERY STIFF, B TRACE GRAVE	BROWN, <b>Silty Clay</b> , Little El, moist	ESAND,				<u> </u>	8 12 16	31	100	SS-3B	3.00	-	-	-	-	-	-	-	-	21	A-6b (V)	-	V9 19 7
@4.5': HARD, [						- 5 -	16 20 24	48	100	SS-4	4.50	1	3	12	26	58	39	23	16	16	A-6b (10)	-	VEDE TO VED TO Y
@6': TRACE SH	HALE FRAGMENTS			981.5	505	- - 7 -	22 22 32	59	100	SS-5	-	-	-	-	-	-	-	1	-	15	A-6b (V)	-	81284 V 772

ABANDONMENT METHODS, MATERIALS, QUANTITIES: AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 238+92, 23' LT. B-055-0-22 SR44 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: PAGE ELEVATION: 984.2 (NAVD88) EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/21/22 END: SAMPLING METHOD: COORD: 726198.8020 N, 2312738.5280 E 12/21/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm (%) GR CS FS SI CL LL PL PΙ WC **AND NOTES** ID (tsf) 984.2 **TOPSOIL - 4 INCHES** 983.9 (480m) SS-1A AGGREGATE - 6 INCHES A-1-b (V) 983.4 9 100 MEDIUM STIFF, BROWN, SILTY CLAY, LITTLE SAND, 1 2 TRACE CRUSHED STONE, TRACE FABRIC, DAMP FILL SS-1B 15 A-6b (V) 982.7 STIFF, BROWN, SANDY SILT, SOME CLAY, TRACE GRAVEL, DAMP 2 100 SS-2 2.00 2 10 19 43 26 26 17 9 16 A-4a (7) 260 981.1 3 SS-3A A-4a (V) STIFF, BROWN/GRAY, SILTY CLAY, LITTLE SAND, MOIST 2 4 100 SS-3B 1.25 25 61 21 AND. 0 4 10 39 18 26 A-6b (11) 4 NOD @4.5': VERY STIFF, GRAY Ø > ₩ 1 L 5 4 13 100 SS-4 3.25 24 A-6b (V) (1830) 978.2 6 VERY STIFF, BROWN/GRAY, SILT AND CLAY, LITTLE SAND, MOIST (ORGANIC ODOR NOTED) 8 100 SS-5 18 3.50 A-6a (V) 976.7 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

X:\PROJECTS\2279801.GPJ

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 242+84, 10' RT. B-056-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE ELEVATION: 980.1 (NAVD88) EOB: PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 7.5 ft. 1 OF 1 START: 12/21/22 END: COORD: 726584.2760 N, 2312812.7580 E 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 980.1 ASPHALT - 3.25 INCHES 979.8 **CONCRETE - 9 INCHES** 979.0 - 1 -BROWN. FINE SAND. TRACE SILT 978.7 HARD, GRAY, SANDY SILT, TRACE CLAY, TRACE SANDSTONE FRAGMENTS, DAMP 2 25 100 SS-1 20 29 42 5 34 25 9 A-4a (2) 100 977.0 3 SS-2A A-4a (V) VERY STIFF, BROWN/GRAY, SILTY CLAY, SOME SAND. TRACE GRAVEL. MOIST 3 10 100 SS-2B 2.00 2 7 23 50 37 20 17 18 23 A-6b (10) THE STATE OF THE S NOOD @4.5': HARD SS-3A 4.50 15 A-6b (V) 975.3 VERY STIFF TO HARD, BROWN, SANDY SILT, LITTLE **₹**3 > **₹**2 5 CLAY, TRACE GRAVEL, DAMP 1 L 12 30 100 SS-3B A-4a (V) 400 WBAN 974.1 6 HARD, BROWN, SILT AND CLAY, SOME SAND, DAMP 1 0 N 35 100 SS-4 16 4.50 16 A-6a (V) 16 972.6 NOTES: NONE

ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

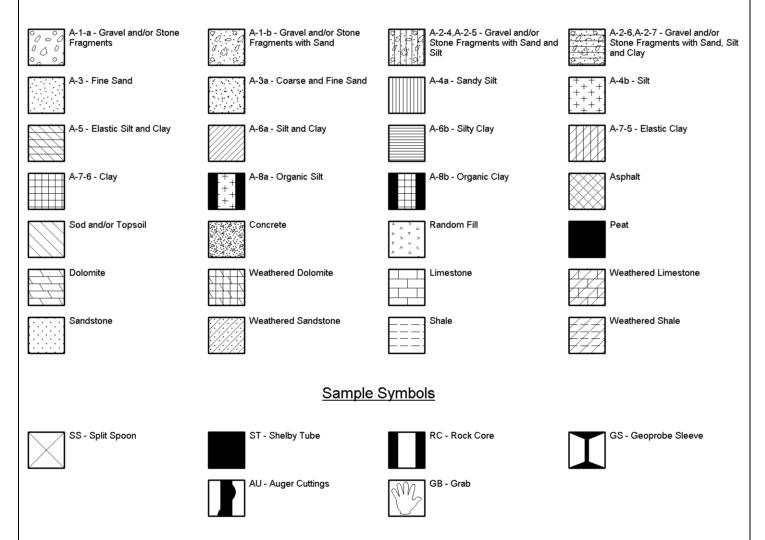
-S\2279801.GPJ

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 246+69, 13' LT. B-057-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION: 977.1 (NAVD88) EOB: 7.5 ft. 1 OF 1 START: 12/21/22 END: COORD: 726970.1180 N, 2312833.6270 E 12/21/22 SAMPLING METHOD: SPT **ENERGY RATIO (%):** 66 ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG MATERIAL DESCRIPTION** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD ppm (%) GR CS FS SI CL LL PL ы WC **AND NOTES** ID (tsf) 977.1 ASPHALT - 4 INCHES 976.8 **CONCRETE - 11 INCHES** 1 975.8 AGGREGATE BASE - 4 INCHES 975.5 SS-1A - -A-1-b (V) A L HARD, BROWN, SANDY SILT, SOME CLAY, TRACE GRAVEL. DAMP 2 18 100 SS-1B 2 7 43 30 17 4.50 18 27 10 13 A-4a (8) 210 10 3 SS-2A 4.50 11 A-4a (V) 2 > V 973.6 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND, 11 28 100 LITTLE SANDSTONE FRAGMENTS. TRACE GRAVEL. 14 SS-2B 22 44 33 21 12 A-6a (7) 14 11 15 DAMP THE STATE OF THE S 972.5 W 000 DENSE. BROWN. GRAVEL AND/OR STONE FRAGMENTS, (SANDSTONE FRAGMENTS), TRACE 5 SILT. DAMP 1 1 46 100 SS-3 A-1-b (V) Sept. WBAN 971.1 6 MEDIUM DENSE. BROWN. FINE SAND. TRACE GRAVEL, TRACE SILT, DAMP 1 0 N 20 22 100 SS-4 10 10 A-3 (V) 10 969.6 -EOB NOTES: NONE ABANDONMENT METHODS, MATERIALS, QUANTITIES: PLACED 0.25 BAG ASPHALT PATCH; AUGER CUTTINGS MIXED WITH 0.25 BAG BENTONITE CHIPS

EXPLORATION ID PROJECT: GEA/LAK-44-18.45/00.00 DRILLING FIRM / OPERATOR: TTL / CW DRILL RIG: CME 75 TRUCK 844 STATION / OFFSET: 250+65, 30' RT. B-058-0-22 TYPE: **ROADWAY** SAMPLING FIRM / LOGGER: TTL / KKC HAMMER: CME AUTOMATIC ALIGNMENT: SR44 PAGE PID: 114163 SFN: N/A DRILLING METHOD: 3.5" SSA CALIBRATION DATE: 3/15/21 ELEVATION: 975.3 (NAVD88) EOB: 1 OF 1 START: 12/21/22 END: SAMPLING METHOD: COORD: 727357.9030 N, 2312925.0320 E 12/21/22 SPT **ENERGY RATIO (%):** 66 **MATERIAL DESCRIPTION** ELEV. REC SAMPLE HP **GRADATION (%) ATTERBERG** SPT/ ODOT SO4 BACK **DEPTHS** CLASS (GI) FILL RQD (tsf) GR CS FS SI ppm (%) CL LL PL PΙ WC **AND NOTES** ID 975.3 ASPHALT - 7 INCHES 974.7 **CONCRETE - 8 INCHES** - 1 -974.0 HARD, GRAY, SANDY SILT, TRACE CRUSHED STONE, TRACE ASPHALT FRAGMENTS, TRACE CLAY, DAMP FILL 2 22 100 SS-1 7 19 27 42 5 32 25 7 A-4a (2) 100 \_ 26 3 SS-2A A-4a (V) 972.0 VERY STIFF, BROWN, SILT AND CLAY, SOME SAND. TRACE GRAVEL, DAMP 4 11 100 SS-2B 2.75 3 9 18 26 44 29 18 11 16 A-6a (7) 4 ALL PARTY @4.6': TRACE SHALE FRAGMENTS N OSD 5 17 100 SS-3 3.50 15 A-6a (V) 7 L 400 969.3 6 STIFF, BROWN/GRAY, SILTY CLAY, LITLTE SAND. (VS) TRACE GRAVEL, TRACE SHALE FRAGMENTS, MOIST 7 12 100 SS-4 1.25 19 A-6b (V) 1 2 2 A L 967.8 NOTES: NONE

### **LEGEND KEY**

### Ohio Department of Transportation Soil Symbols



#### Notes:

- 1. Pavement cores and exploratory borings were performed during the period from December 12 through 22, 2022. Cores were obtained using a nominal 4-inch diameter core barrel. Borings were advanced using 3½-inch diameter solid-stem augers.
- 2. These logs are subject to the limitations, conclusions, and recommendations in the report and should not be interpreted separate from the report.
- 3. The boring locations were established in the field by TTL Associates, Inc. in general accordance with plans provided with the proposal for this project.



2

67

2

25

23

22

3.62

3.65

34.81

33.43

PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-003-0-22

B-003-0-22

B-004-0-22

•

1.5

4.1

1.5

1.547

0.072

1.578

0.278

0.337

0.125

0.162

0.011

0.015

4

2

4

33

3

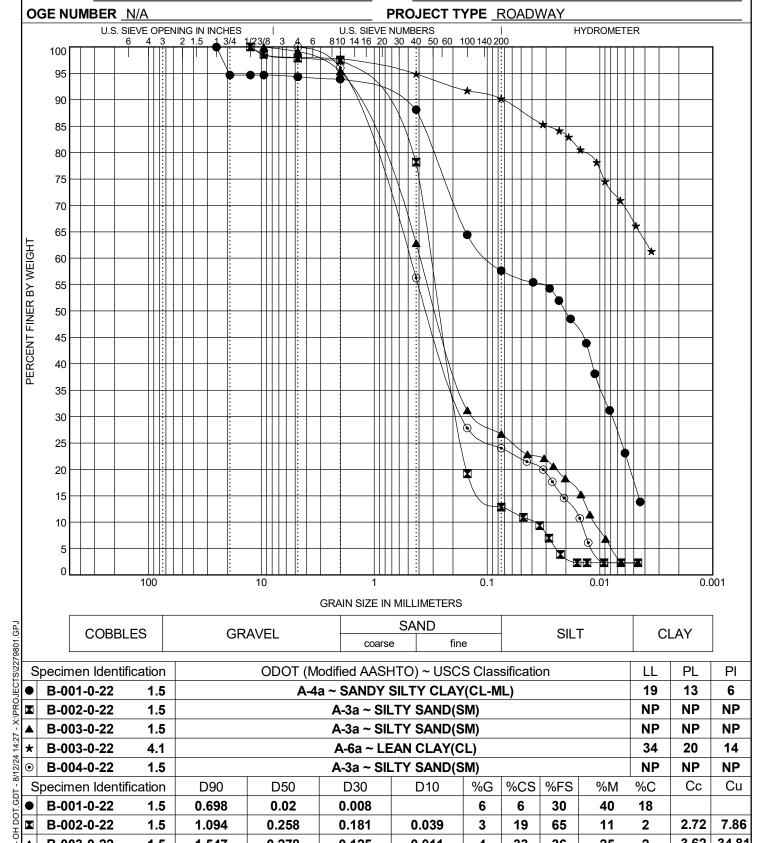
40

36

5

32

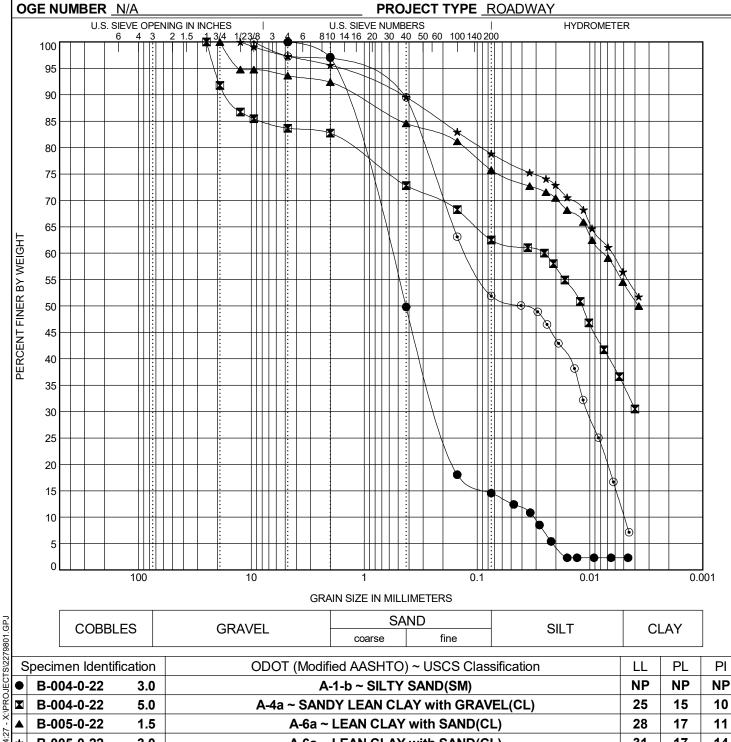
**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



7980	l
ECTS/22	Ī
ECT	ŀ
:\PROJI	ŀ
-X:\F	ŀ
4:27	ŀ
12/24 1	ŀ
8/12	ŀ
3DT-	ŀ
DOT.GDT	ŀ
OHD	Ļ
- 1	L
N SIZE	
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בַּוַנַ	<u> </u>	D-004-0-22	5.0	A-4a ~ SANDY LEAN CLAY WITH GRAVEL(CL)										10	
×.	▲	B-005-0-22	1.5		A-6a ~ LEAN CLAY with SAND(CL)										
4 14:2	*	B-005-0-22	3.0		A-6a ~ LEAN CLAY with SAND(CL)										
12/2	•	B-006-0-22	1.5		A-4a ~ SANDY SILTY CLAY(CL-ML)										
8-	Sp	oecimen Identif	fication	D90	D50	D30	D10	%G	%CS	%FS	%M	%C	Сс	Cu	
<u> </u>		B-004-0-22	3.0	1.583	0.427	0.222	0.031	3	47	35	13	2	2.63	18.82	
	X	B-004-0-22	5.0	16.4	0.012			17	10	10	28	35			
4-c	<b>A</b>	B-005-0-22	1.5	1.242				7	8	9	22	54			
ກ∣.	<b>.</b> ⊢	B-005-0-22	3.0	0.471				4	6	11	23	56			
GKAIN GKAIN	<b>9</b>	B-006-0-22	1.5	0.467	0.04	0.01	0.005	3	7	38	42	10	0.18	24.96	



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-007-0-22

B-008-0-22

•

0.85

1.636

3.5

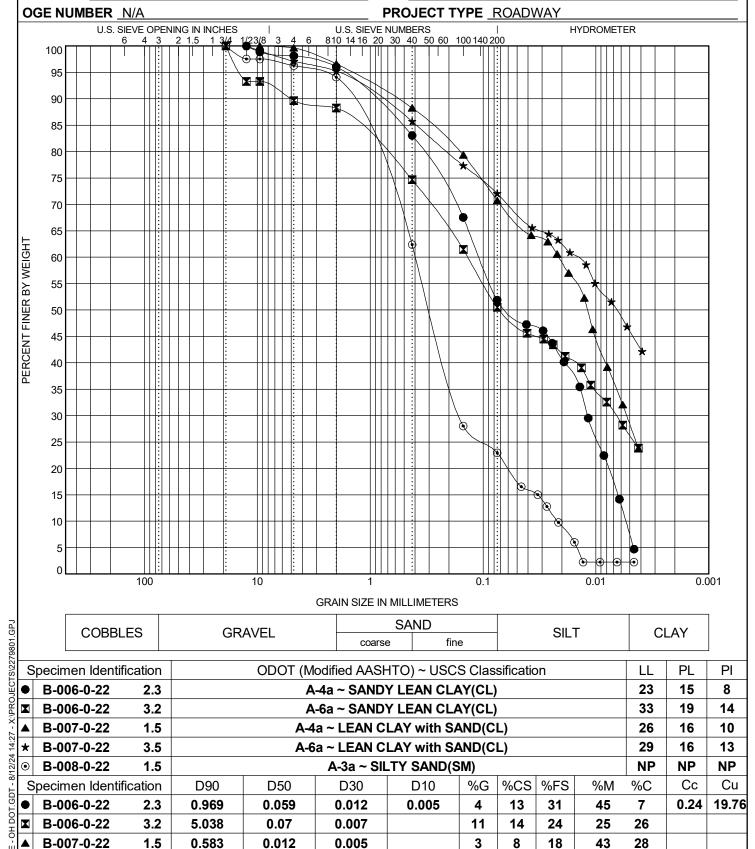
1.5

0.007

0.292

0.159

**PID** 114163



4

6

0.022

10

32

14

39

26

21

46

2

2.93

18.07

17

18

23

50

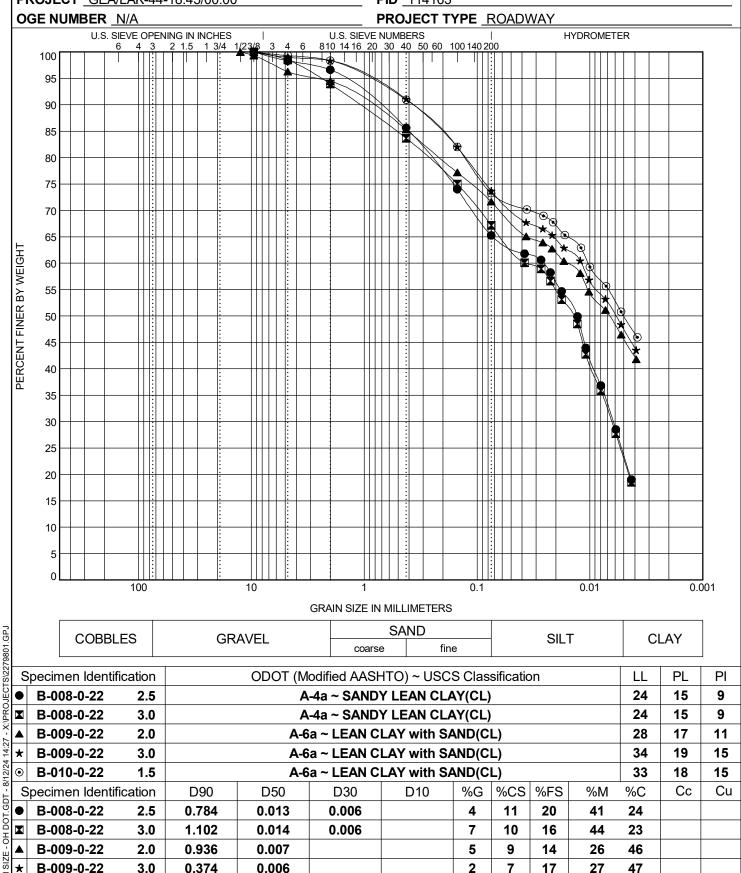
2

7



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



•

B-010-0-22

3.0

1.5

0.379

0.005



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-012-0-22

B-012-0-22

•

1.5

3.0

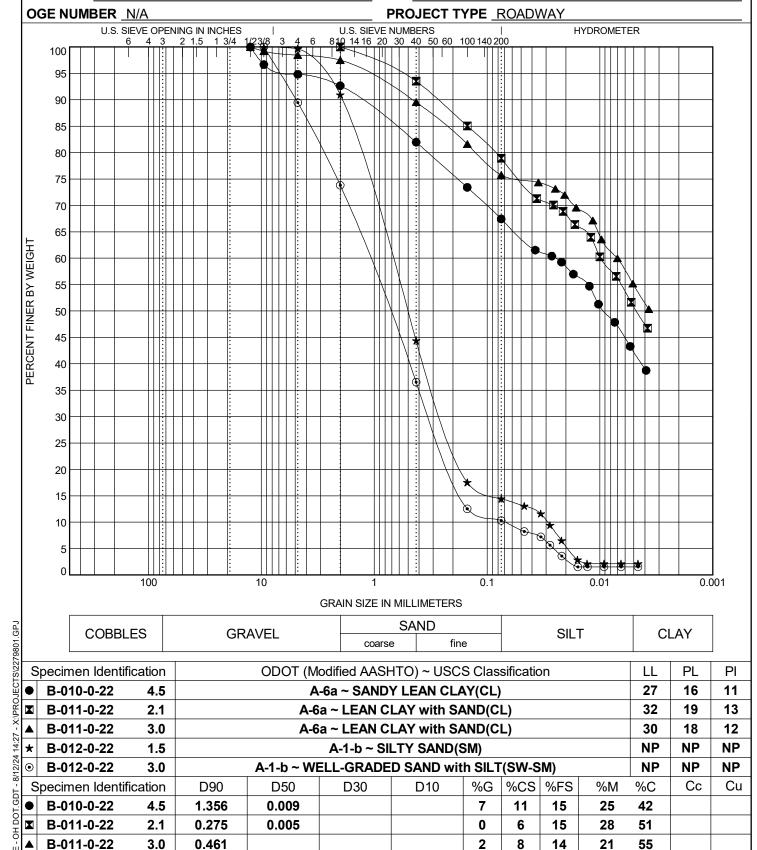
1.937

4.917

0.512

0.743

**PID** 114163



0.029

0.069

9

27

47

37

30

26

2

2

12

8

2.85

1.31

24.58

16.20

0.243

0.32



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-019-0-22

B-019-0-22

•

1.7

3.0

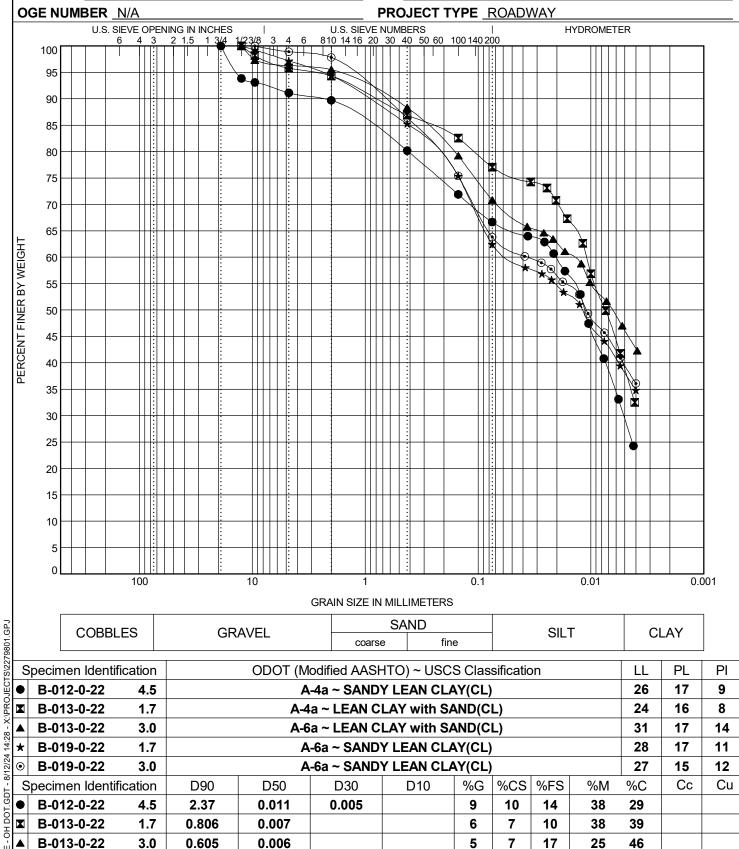
0.947

0.691

0.012

0.011

**PID** 114163



6

2

9

11

23

23

24

25

38

39



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-021-0-22

B-022-0-22

•

4.5

1.7

22.185

1.696

0.092

0.008

0.007

23

9

10

8

20

13

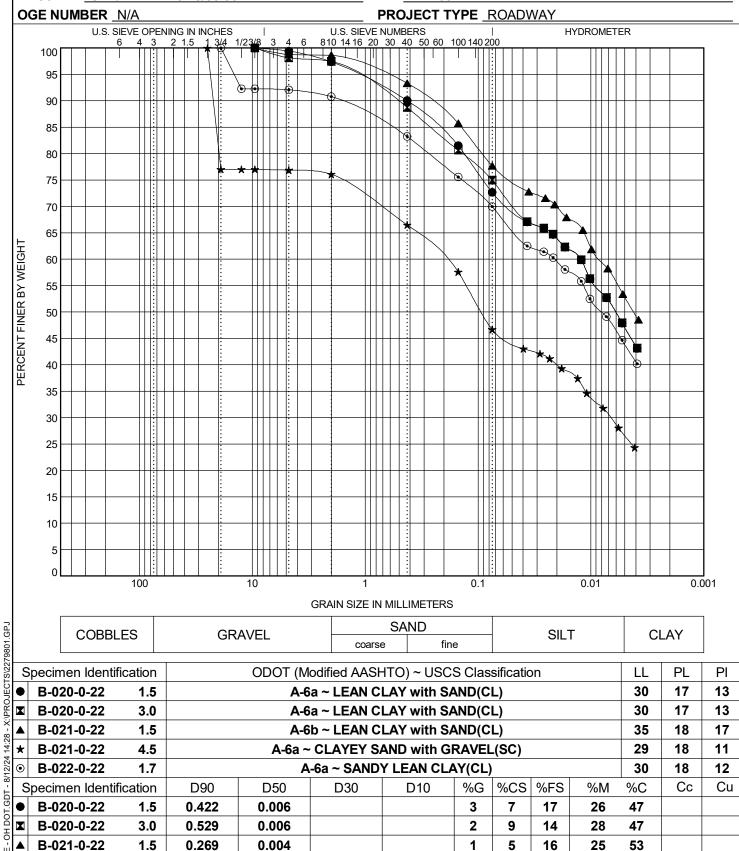
20

26

27

44

**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-024-0-22

B-024-0-22

•

1.5

3.0

2.343

1.66

0.526

0.395

0.278

0.156

0.152

0.019

12

5

44

44

37

25

5

22

2

4

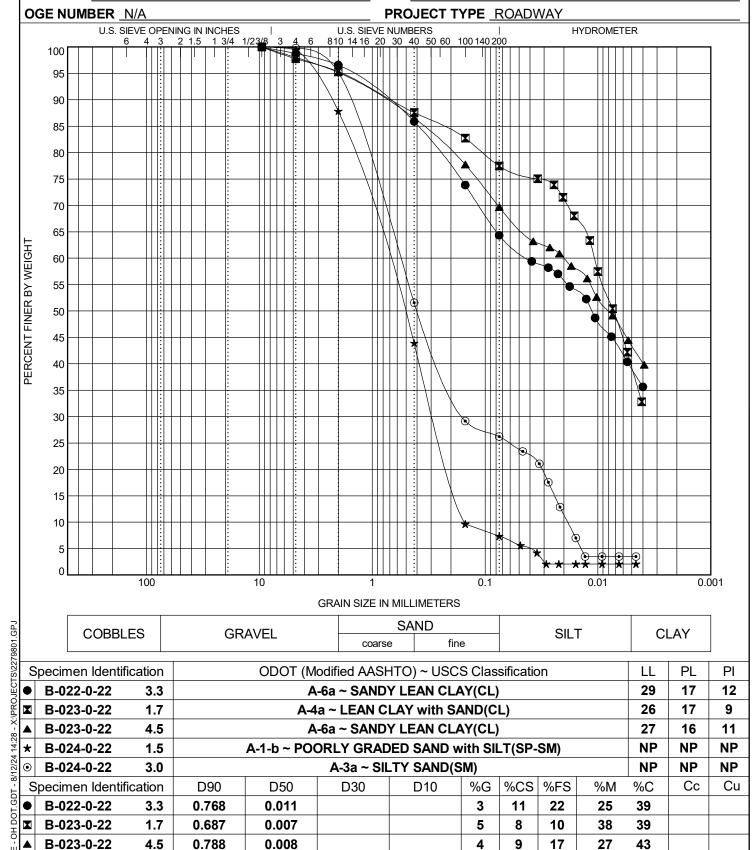
0.68

2.29

4.94

30.96

**PID** 114163



18

31

33

%C

4

10

58

59

%M

22

35

21

25

1

16

18

19

Cc **1.24** 

2.29

0.10

2

13

**14** Cu

2.97

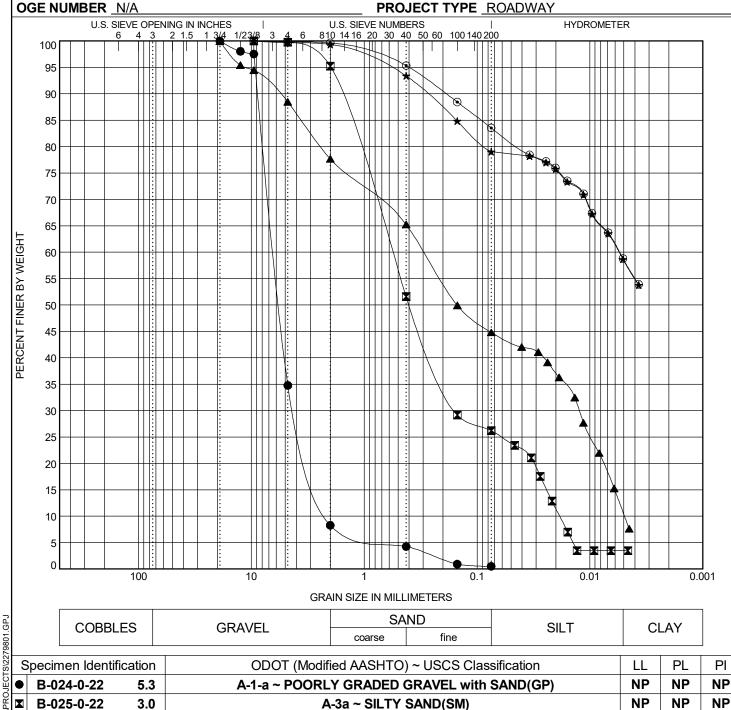
30.96

60.13



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



A-4a ~ SILTY SAND(SM)

A-6a ~ LEAN CLAY with SAND(CL)

A-6a ~ LEAN CLAY with SAND(CL)

D10

2.115

0.019

0.005

%G

91

5

23

1

0

%CS

4

44

12

6

4

%FS

4

25

20

14

12

D30

4.063

0.156

0.012

B-025-0-22

B-026-0-22

B-026-0-22

B-024-0-22

B-025-0-22

B-025-0-22

B-026-0-22

B-026-0-22

Specimen Identification

5.1

1.8

3.0

5.3

3.0

5.1

1.8

3.0

D90

8.74

1.66

5.659

0.281

0.189

D50

5.619

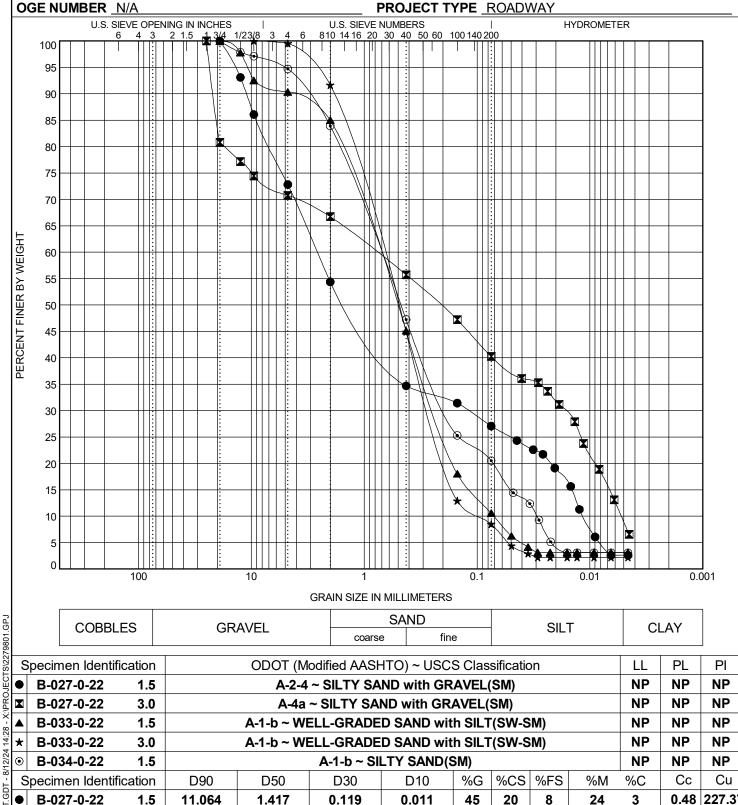
0.395

0.15



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



OH DOT

 $\blacksquare$ 

•

B-027-0-22

B-027-0-22

B-033-0-22

B-033-0-22

B-034-0-22

1.5

3.0

1.5

3.0

1.5

11.064

21.67

4.484

1.892

3.251

1.417

0.21

0.514

0.503

0.477

0.119

0.017

0.238

0.262

0.187

0.011

0.005

0.071

0.095

0.03

45

33

14

8

15

20

11

40

47

37

8

16

35

36

27

24

31

8

7

18

3

9

3

2

3

0.07 146.64

10.69

7.40

24.64

1.05

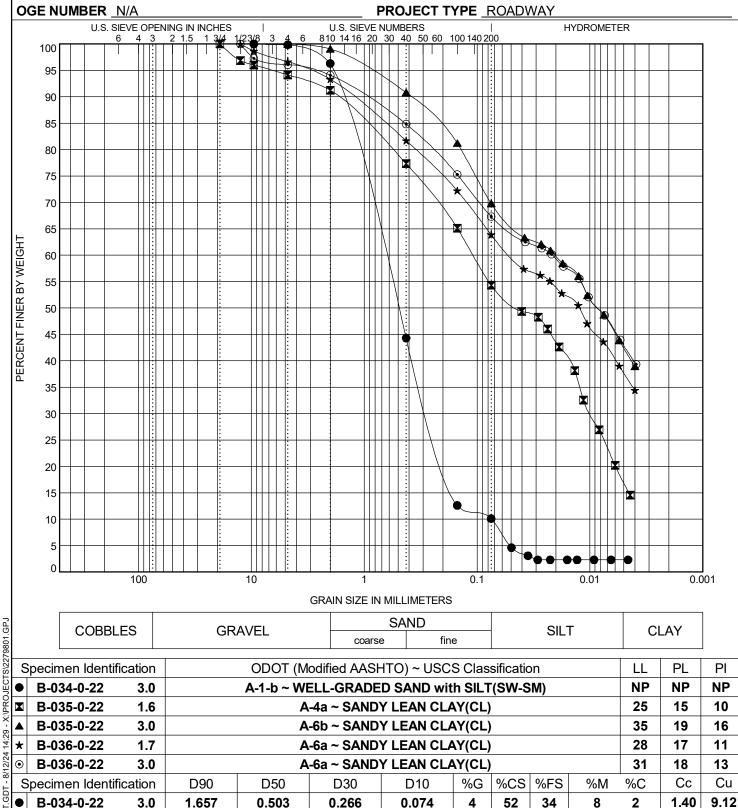
1.03

1.63



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



- OH DOT

 $\blacksquare$ 

•

B-034-0-22

B-035-0-22

B-035-0-22

B-036-0-22

B-036-0-22

3.0

1.6

3.0

1.7

3.0

1.657

1.739

0.388

1.28

1.006

0.503

0.043

0.008

0.012

0.008

0.266

0.01

0.074

4

9

1

6

6

52

14

8

12

9

34

23

21

18

18

8

37

28

26

24

17

42

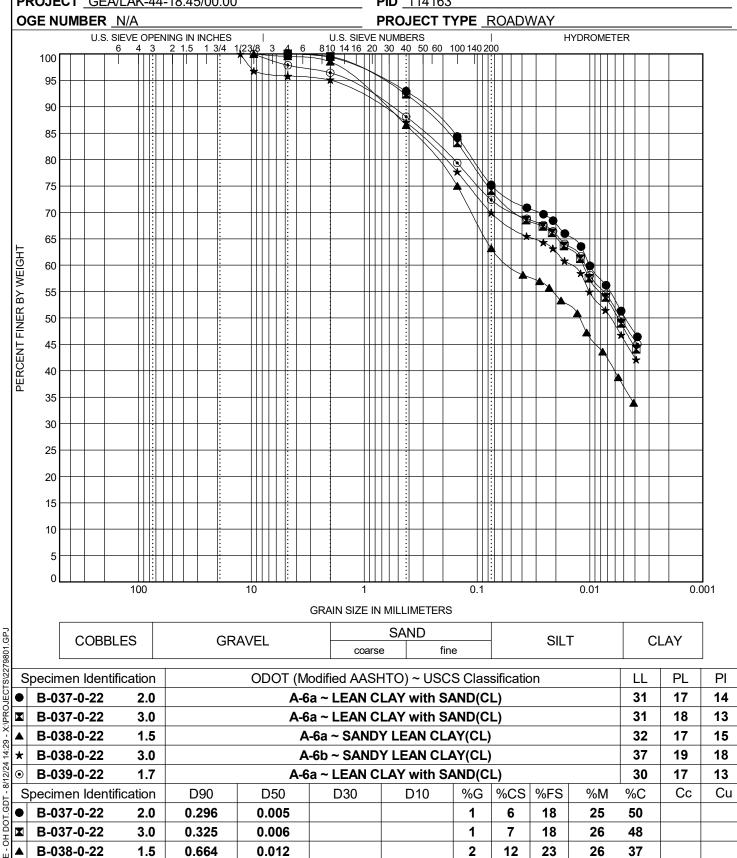
38

43



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



5

4

8

8

17

16

24

23

46

49

•

B-038-0-22

B-039-0-22

3.0

1.7

0.747

0.6

0.007

0.005

32.67

32.80

2.73

2.81



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-041-0-22

B-041-0-22

•

1.5

3.0

1.853

3.502

0.369

0.536

0.16

0.226

0.017

0.024

39

42

8

15

29

25

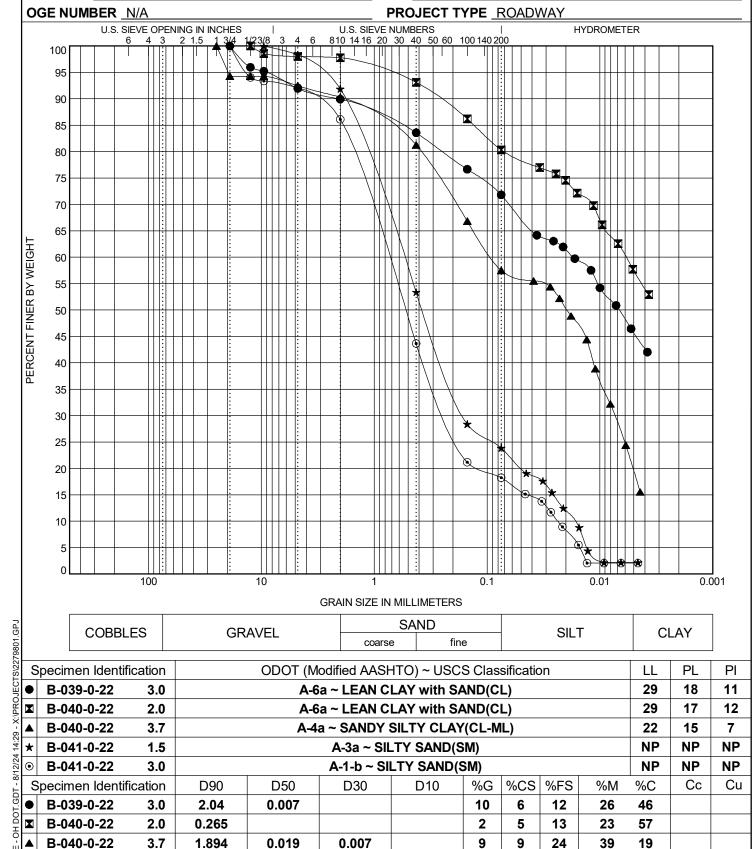
22

16

2

2

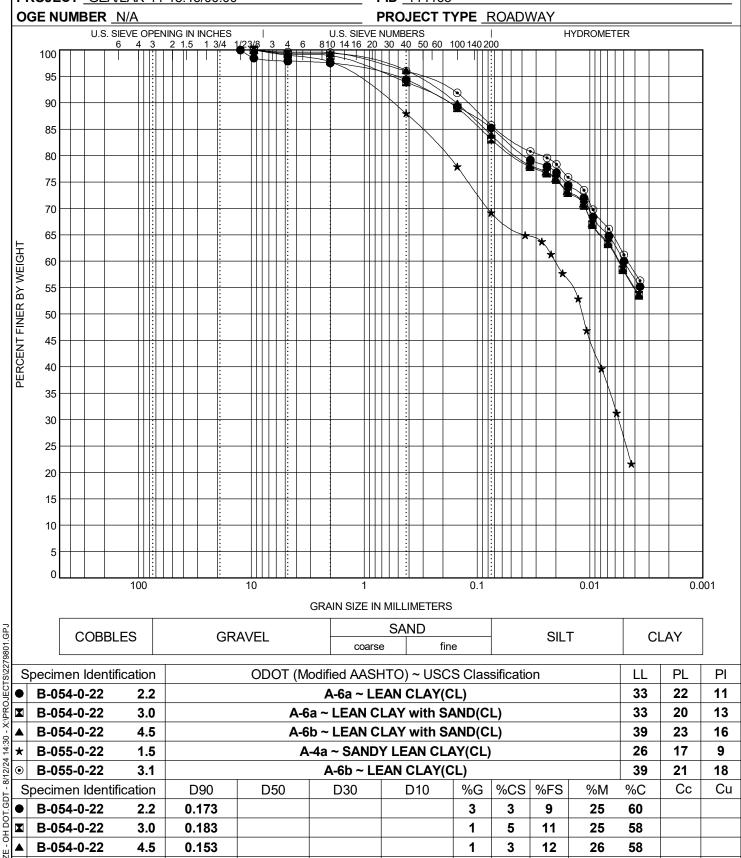
**PID** 114163





PROJECT <u>GEA/LAK-44-18.45/00.00</u>

**PID** 114163



14:30 -

•

B-055-0-22

B-055-0-22

1.5

3.1

0.584

0.121

0.012

0.006

2

0

10

4

19

10

43

25

26

61



PROJECT <u>GEA/LAK-44-18.45/00.00</u>

B-057-0-22

B-058-0-22

•

8.286

1.576

3.5

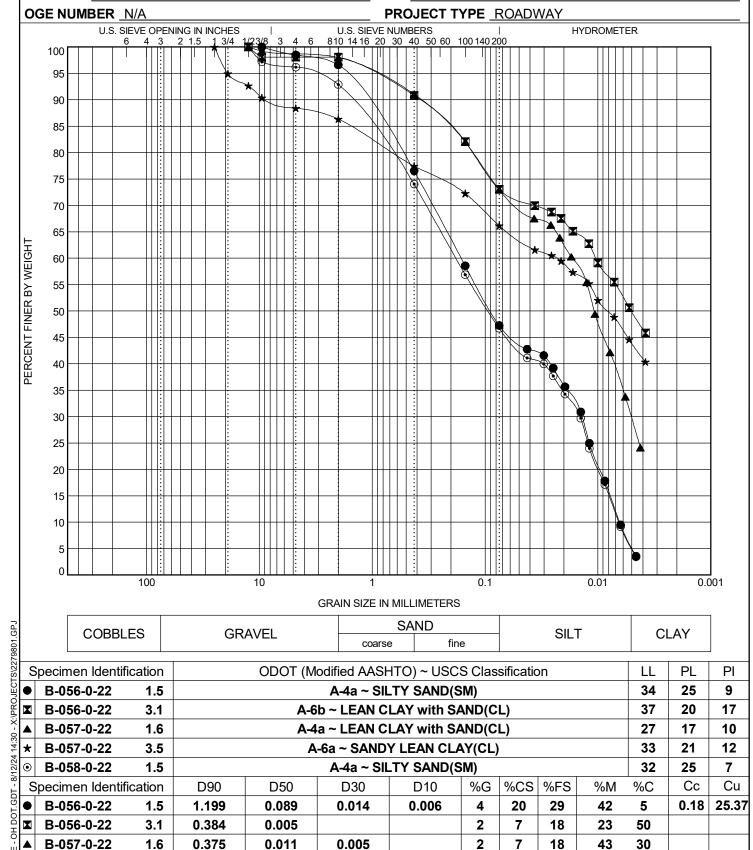
1.5

0.008

0.094

0.014

**PID** 114163



14

7

0.007

9

19

11

27

22

42

44

5

0.18 27.78



PROJECT <u>GEA/LAK-44-18.45/00.00</u> PID <u>114163</u>

NCHES 1 3/4 1/23/8 3 4 6 OGE NUMBER N/A PROJECT TYPE ROADWAY U.S. SIEVE OPENING IN INCHES U.S. SIEVE NUMBERS HYDROMETER 810 14 16 20 30 40 50 60 100 140 200 100 95 90 85 80 75 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 100 10 0.01 0.001 **GRAIN SIZE IN MILLIMETERS** SAND 'S\2279801.GPJ **COBBLES GRAVEL** SILT **CLAY** fine coarse

<u>`</u> .L												
LS/22	Specimen Identification		ODOT (M	odified AASI	HTO) ~ USC	S Clas	sificatio	on		LL	PL	Pl
- X:\PROJECTS\227	● B-058-0-22 3.3		Α	-6a ~ SAND	Y LEAN CL	AY(CL)	)			29	18	11
PRO												
×												
14:3												
12/24												
- OH DOT.GDT - 8/12/24 14:30	Specimen Identification	D90	D50	D30	D10	%G	%CS	%FS	%M	%C	Сс	Cu
T.GD	● B-058-0-22 3.3	0.594	0.007			3	9	18	26	44		
띩												
SIZ												
SRAIN SIZE												



### **Core Log For B-001-0-22**

Project: Proposed Intersection Improvements

Project Location : Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 12, 2022





ASPHALT THICKNESS (in.)	=	6
CONCRETE THICKNESS (in.)	=	8
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement cores appeared in good condition.

Rebar encountered at 4.5" from top of concrete

No apparent aggregate base



### **Core Log For B-010-0-22**

Project: Proposed Intersection Improvements

Project Location:Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 14, 2022





ASPHALT THICKNESS (in.)	=	6.5
CONCRETE THICKNESS (in.)	=	8
SAND BASE THICKNESS (in.)	=	2
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement cores appeared in good condition.

Rebar encountered at approximately 4 inches below top of pavement



# **Core Log For B-013-0-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 14, 2022





ASPHALT THICKNESS (in.)	=	6.5
CONCRETE THICKNESS (in.)	=	8.5
AGGREGATE BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement cores appeared in good condition.			



# Core Log For B-020-0-22

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 15, 2022



ASPHALT THICKNESS (in.)	=	8.5
AGGREGATE BASE THICKNESS (in.)	=	8.5
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement core appeared in good condition.			



### **Core Log For B-026-0-22**

Project: Proposed Intersection Improvements

Project Location:Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 15, 2022





ASPHALT THICKNESS (in.)	=	6.5
CONCRETE THICKNESS (in.)	=	9
AGGREGATE BASE THICKNESS (in.)	=	6
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Asphalt core appeared in good condition.

Horizontal fracture in concrete core at approximately 3.5 inches below top concrete.



### **Core Log For B-038-0-22**

Project : Proposed Intersection Improvements

Project Location: Chardon & Concord Townships, Geauga & Lake Counties, Ohio

TTL Project No. 2279801 Core Date: December 12, 2022





ASPHALT THICKNESS (in.)	=	4.5
CONCRETE THICKNESS (in.)	=	9
SAND BASE THICKNESS (in.)	=	5
CORE BARREL DIAMETER (in.)	=	4

#### **VISUAL DESCRIPTION:**

Pavement cores appeared in good condition.				
Rebar encountered at approximately 5 inches below top of concrete				

#### APPENDIX C

(Geotechnical Engineering Design Checklists)



I. Geotechnical Design Checklists				
Project: GEA/LAK-44-18.45/00.00	PDP Path:			
PID: 114163	Review Stage: 1			

Checklist	Included in This Submission
II. Reconnaissance and Planning	✓
III. A. Centerline Cuts	
III. B. Embankments	
III. C. Subgrade	✓
IV. A. Foundations of Structures	
IV. B. Retaining Wall	
V. A. Landslide Remediation	
V. B. Rockfall Remediation	
V. C. Wetland or Peat Remediation	
V. D. Underground Mine Remediation	
V. E. Surface Mine Remediation	
V. F. Karst Remediation	
VI. A. Soil Profile	
VI. D. Geotechnical Reports	✓

# II. Reconnaissance and Planning Checklist

C-R-S:	GEA/LAK-44-18.45/00.00	PID: 114163	Reviewer	: CPI	Date:	2/28/2023
Reconn	naissance		(Y/N/X)	Notes:		
1	Based on Section 302.1 in the necessary plans been develop areas prior to the commencer subsurface exploration reconn	ed in the following ment of the	X	Plans to be prep	ared by othe	Prs.
	Roadway plans Structures plans Geohazards plans			<u> </u>  -		
2	Have the resources listed in So the SGE been reviewed as par reconnaissance?		Y			
3	Have all the features listed in the SGE been observed and ex- field reconnaissance?		Υ			
4	If notable features were disco reconnaissance, were the GPS these features recorded?		Х			
Dlannir	ng - General		(Y/N/X)	Notes:		
5	In planning the geotechnical e program for the project, have geologic conditions, the propo- historic subsurface exploration considered?	the specific osed work, and	Y	Notes.		
6	Has the ODOT Transportation Mapping System (TIMS) been available historic boring inform inventoried geohazards?	accessed to find all mation and	Υ			
7	Have the borings been located maximum subsurface informa minimum number of borings, geotechnical explorations to t possible?	tion while using a utilizing historic	Y			
8	Have the topography, geologic materials, surface manifestatic conditions, and any other speconsiderations been utilized in spacing and depth of borings?	on of soil cial design n determining the	Y			
9	Have the borings been located adequate overhead clearance equipment, clearance of unde minimize damage to private p minimize disruption of traffic, compromising the quality of t	for the erground utilities, roperty, and without	Y			

# II. Reconnaissance and Planning Checklist

Planni	ng - General	(Y/N/X)	Notes:
10	Have the scaled boring plans, showing all project and historic borings, and a schedule of borings in tabular format, been submitted to the District Geotechnical Engineer?	Υ	Provided as part of proposal for this project.
	The schedule of borings should present the follow information for each boring:	ving	
а	. exploration identification number	Υ	
b	. location by station and offset	Χ	Not provided.
С	estimated amount of rock and soil, including the total for each for the entire program.	Υ	
Planni	ng – Exploration Number	(Y/N/X)	Notes:
11	Have the coordinates, stations and offsets of all explorations (borings, probes, test pits, etc.) been identified?	Х	Not provided.
12	Has each exploration been assigned a unique identification number, in the following format X-ZZZ-W-YY, as per Section 303.2 of the SGE?	Υ	
13	When referring to historic explorations that did not use the identification scheme in 12 above, have the historic explorations been assigned identification numbers according to Section 303.2 of the SGE?	Х	No specific borings were referenced or utilized, since no SPT data was available. Generalized encountered conditions were summarized in the report.

# II. Reconnaissance and Planning Checklist

Planni	ng – Boring Types	(Y/N/X)	Notes:
14	Based on Sections 303.3 to 303.7.6 of the SGE,		
	have the location, depth, and sampling	Υ	
	requirements for the following boring types	Y	
	been determined for the project?		
	Check all boring types utilized for this project:		
	Existing Subgrades (Type A)	<b>√</b>	
	Roadway Borings (Type B)	✓	
	Embankment Foundations (Type B1)		
	Cut Sections (Type B2)		
	Sidehill Cut Sections (Type B3)		
	Sidehill Cut-Fill Sections (Type B4)		
	Sidehill Fill Sections on Unstable Slopes (Type		
	B5)		
	Geohazard Borings (Type C)		
	Lakes, Ponds, and Low-Lying Areas (Type C1)		
	Peat Deposits, Compressible Soils, and Low		
	Strength Soils (Type C2)		
	Uncontrolled Fills, Waste Pits, and Reclaimed		
	Surface Mines (Type C3)		
	Underground Mines (C4)		
	Landslides (Type C5)		
	Rockfall (Type C6)		
	Karst (Type C7)		
	Proposed Underground Utilities (Type D)		
	Structure Borings (Type E)		
	Bridges (Type E1)		
	Culverts (Type E2 a,b,c)		
	Retaining Walls (Type E3 a,b,c)		
	Noise Barrier (Type E4)		
	CCTV & High Mast Lighting Towers		
	(Type E5)		
	Buildings and Salt Domes (Type E6)		

# III.C. Subgrade Checklist

C-R-S:	GEA/LAK-44-18.45/00.00	PID:	114163	Reviewer:	CPI	Date:	2/28/2023
	If you do not have any su	bgrade	work on the	e project, you	u do not have to fi	ill out this a	checklist.
Subgra	de			(Y/N/X)	Notes:		
1	Has the subsurface exploration characterized the soil or rock of Geotechnical Bulletin 1: Plan S	accordii	ng to	Y			
a.	Has each sample been visua inspected for the presence of moisture content been performantly sample?	of gypsu	ım? Has a	Y			
b.	Has mechanical classification Liquid Limit (LL), and gradati done on at least two sample within six feet of the propos	on testi	ing) been each boring				
C.	Has the sulfate content of at from each boring within 3 fe subgrade been determined, 1122, Determining Sulfate C	et of th per Sup	ne proposed oplement	Y			
d.	Has the sulfate content of al exhibit gypsum crystals beer	•		Х	No gypsum obser	ved in sam	ples.
e.	Have A-2-5, A-4b, A-5, A-7-5 within the top 3 feet of the peen mechanically classified	oropose			None present.		
2	If soils classified as A-2-5, A-4b or A-8b, or having a LL>65, are proposed subgrade (soil profil specify that these materials no and replaced or chemically sta	o, A-5, A e preser e), do t eed to b	nt at the he plans be removed	Х	None present.		
a.	If these materials are to be replaced, have the station line lateral limits for the planned provided?	mits, de	epth, and	Х			
3	If there is any rock, shale, or control proposed subgrade (C&MS 20) specify the removal of the ma	4.05), d		Х	Rock not present widening areas.	at subgrad	e elevations in
a.	If removal of any rock, shale required, have the station lin lateral limits for the planned material at proposed subgra	mits, de I remov	epth, and al of the	Х			

#### III.C. Subgrade Checklist

	do	(\/ /N   /\/)	Notos
Subgra		(Y/N/X)	Notes:
4	In accordance with GB1, do the SPT (N <sub>60</sub> )/HP values and existing moisture contents for the proposed subgrade soils indicate the need for subgrade stabilization?	Υ	
a.	If removal and replacement is applicable, has the detail of subgrade removal been shown on the plans, including depth of removal, station limits, lateral extent, replacement material, and plan notes (Item 204 - Subgrade Compaction and Proof Rolling)?	Υ	
b.	If chemical stabilization is applicable, has the detail of this treatment been shown on the plans, including depth, percentage of chemical, station limits, lateral extent, and plan notes?	X	Cement stabilization is option per Subgrade Analysis spreadsheet, but widening is generally 12 feet or less in width. Therefore, undercut and replacement with granular engineered fill is recommended.
	Indicate type of chemical stabilization specified:		
	cement stabilization	<b>√</b>	7
	lime stabilization		- -
5	If removal and replacement has been specified, do the plans include Plan Note G121 from L&D3?	Х	Plans to be prepared by others.
6	If drainage or groundwater is an issue with the proposed subgrade, has an appropriate drainage system (e.g., pipe, underdrains) been provided?	X	Plans to be prepared by others.
7	Has an appropriate quantity of Proof Rolling (C&MS 204.06) and has Plan Note G111 from L&D3 been included in the plans?	Х	Plans to be prepared by others.
8	Has a design CBR value been provided?	Υ	

# VI.B. Geotechnical Reports

C-R-S:	GEA/LAK-44-18.45/00.00	PID:	114163	Reviewer:	СРІ	Date:	8/12/2024
General				(Y/N/X)	Notes:		
:	Has an electronic copy of all g submissions been provided to Geotechnical Engineer (DGE)?	the Distr		X	This submittal is Consultant, who		
	Has the first complete version report being submitted been I	J		Υ			
	Subsequent to ODOT's review the complete version of the re report being submitted been I	evised ged	otechnical	Υ	This is the final re	eport submi	ttal
	Has the boring data been subr format that is DIGGS (Data Int Geotechnical and Geoenviron compatable? gINT files may be	erchange mental)	for	Υ	Providing gINT pr	roject file wi	th this submmital.
	Does the report cover format Brand and Identity Guidelines found at http://www.dot.stato oh.us/brand/Pages/default.as	Report Ste. e. px ?	tandards	Υ			
	Have all geotechnical reports been titled correctly as prescr 705.1 of the SGE?	-		Υ			
Report I				(Y/N/X)	Notes:		
	Do all geotechnical reports be contain the following:						
a.	an Executive Summary as de 705.2 of the SGE?	escribed in	1 Section	Υ			
b.	an Introduction as described of the SGE?			Υ			
C.	a section titled "Geology and the Project," as described in the SGE?			Υ			
d.	a section titled "Exploration, Section 705.5 of the SGE?	," as desc	ribed in	Υ			
e.	a section titled "Findings," as Section 705.6 of the SGE?	s describe	ed in	Υ			
f.	a section titled "Analyses an Recommendations," as desc 705.7 of the SGE?		ection	Υ			
Append				(Y/N/X)	Notes:		
	Do all geotechnical reports be contain all applicable Appendi Section 705.8 of the SGE?	Ū		Υ			
9	Do the Appendices present as showing all boring locations as Section 705.8.1 of the SGE?		-	Υ	Boring numeration	on shortene	d for clarity.

#### VI.B. Geotechnical Reports

Apper	ndices	(Y/N/X)	Notes:						
10	Do the Appendices include boring logs and color pictures of rock, if applicable, as described in Section 705.8.2 of the SGE?	Х	No rock cored.						
11	Do the Appendices include reports of undisturbed test data as described in Section 705.8.3 of the SGE?	Х	No undisturbed tests.						
12	Do the Appendices include calculations in a logical format to support recommendations as described in Section 705.8.4 of the SGE?	Υ							

#### APPENDIX D

(Historic Borings)



#### GENERAL INFURMATION

#### INTRODUCTION

The project consists of a major realignment of SR-44, and associated intersecting roads - Mentor Rd. (West), Hosford Rd., Clark Rd., Colburn Rd., and Girdled Rd. - approximately 5.2 miles in length, beginning approximately 1.5 miles northwest of Chardon on Mentor Road, Geauga County, extends in a northerly direction, and terminates 300 feet north of Auburn Road, approximately 2.5 miles south of SR-84, Lake County.

The proposed grade indicates the following:

Mainline - cuts, ranging between 0 and 27 feet in depth at ditchline, and fill embankment ranging between 0 and 30 feet in height.

Mentor Road (West) - fill embankment, ranging between 0 and 28 feet in height.

Hosford Road - fill embankment, ranging between 0 and 14 feet in height.

Clark Road - fill embankment, ranging between 0 and 3 feet in height.

Colburn Road - cuts, ranging between 0 and 5 feet in depth.

Girdled Road - fill embankment, ranging between 0 and 14 feet in height.

#### GEOLOGY AND OBSERVATIONS OF THE PROJECT

The alignment originates on an upland plateau region, crosses a bedrock outlier and descends to a lower, relatively flat, glaciated portion of an upland plain region, where the project terminates. Several exposures of bedrock were observed and measured. It is noted that the area is extensively dissected by relatively shallow drainage courses. Glacial drift, ranging in depth from 0 to at least 20 feet, overlies bedrock, comprised of shales and the Sharon Conglomerate, Pennsylvanjan age, in the initial one-third of the project, and shales and sandstones of the Cuyahoga formation, Mississippian age, in the remaining two-thirds.

#### EXPLORATION

Exploratory borings were made by means of truck-mounted mechanical earth auger and hand auger (in areas of difficult access), between April 6 and May 2, 1961.

#### INVESTIGATIONAL FINDINGS

Materials occurring immediately below proposed grade and in the embankment foundation areas are predominantly comprised of sandy silts and silt clays, in the A-4a and A-6a classifications, generally having moisture contents within the plastic range. Between stations 487+00 and 493+00, proposed grade and left backslope will be in sandstone conglomerate. Frost susceptible silts were found to occur within three feet of proposed grade at stations 14+10, 482+50, 483+43, 485+35, 520+50, 580+00, 650+00, 653+90 in Geauga County, and station 11+00 in Lake County.

Wet, soft compressible sediments as much as 4 feet in thickness, were found to occur at-surface between approximately stations 22+25 and 478+50, 609+00 to 616+00, 620+00 to 623+00, and in the majority of the drainage channels throughout the project.

Intersecting Roads - Mentor Road (West), Hosford Road, Clark Road, Colburn Road, and Girdled Road. Materials occurring immediately below proposed grade and in the embankment foundation areas are predominantly comprised of sandy silt and silt clays, in the A-Ma and A-Ga classifications. Frost susceptible silts were found to occur within three feet of proposed grade at stations 52+00 - Mentor Road (West); 7+00 - Colburn Road; and 0+50 - Girdled Road.

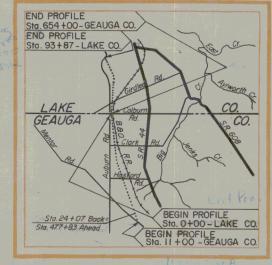
Crave  and  or stone fragments	prominentous														
### Clay   A-1-b (10)   A-1-b   10   13   19   9   10   NP   NP   13   6	LE	GEND FOR PROJE						OF T	man with a see				Obtained Security 2 Decision Security		
Fine sand A-1-b(0) A-1-b 10 13 19 0 10 NP NP 13 6  Fine sand A-3(0) A-3 0 1 60 -10 - NP NP 12 1  Coarse and fine sand A-3a 8 10 56 13 13 NP NP 17 5  Coarse and fine sand silt A-2-1(0) A-2-1 15 6 16 16 15 27 11 21 13  Store fragments with sand and silt A-2-1(0) A-2-1 15 6 16 16 15 27 11 21 13  Store fragments with sand lary A-2-6 (0) A-2-6 13 12 15 13 17 31 11 15 2  Standy silt A-1(1) A-1a 16 8 17 31 26 24 5 18 100  Standy silt A-1(1) A-1a 16 8 17 31 26 24 5 18 100  Stift and clay a-6 (8) A-5 0 3 28 44 25 46 NP 13 1  Stift and clay A-6 (8) A-6 13 5 11 33 38 30 12 19 215  Stift and clay A-7-6 (12) A-7-6 6 1 13 37 52 13 18 29 7  Fouldery ione  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Nater content pearly equal to or greater than liquid limit.  Auger boring plotted to vertical scale only.  **Nater content pearly equal to or greater than liquid limit.  **Horizona to the repair of the standard	000	Gravel and/or stone fragments	A-1-a (0)	A-1-a	66	9	13	7	5	NP	NP	18	3		
Coarse and fine sand	7000	Gravel and/or stone fragments with sand	A-1+b (0)	A-1-b	719	13	19	9	10	NP	NP	13	6		
Gravel and/or stone frag- ments with sand and silt  A-2-li (0) A-2-li 15 6 18 16 15 27 1 21 13  Stone fragments with sand and silt  A-2-6 (0) A-2-6 13 12 15 13 17 31 11 15 2  Sandy silt  A-1 (1) A-1 (1) A-1 (1) 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	FS	#Fine sand	A-3(0)	A-3	o	1.	89	- 1	0:-	NP	NP	22	-		
Sign ents with sand and sit			-	A-3a	8	10/	56	13	13	NP	NP	17	5		
### Sandy silt   A-1 (1)		Gravel and/orestone frag- ments with sand and silt	A-2-4 (0)	A-2-4	45	6	18	16	15	'27	14	21	13		
Silt	000	Stone fragments with sand, silt, and clay	A-2-6 (0)	A+2-6	143	12	15	13	17	<sup>4</sup> 31	11	15	'2		
		Sandy silt	V=31 (31)	A-4a	18	18	17	:31	126	574	15	18	109		
Silt and clay	9	Silt	(8)#≟A	A-4b	ц	γĦ	8	53	31	*29	5	21	8		
Silty clay  A-6 (II) A-6b 8 3 7 38 141 36 17 25 10  Clay  A-7-6 (I2) A-7-6 6 1 11 37 52 13 18 29 7  Fine-Textured peat  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Various other materials  Visual CLASSIFICATION  Visual CLASSIFICATION  Auger boring - plan view.  Auger boring plotted to vertical scale only.  B 3 7 38 141 36 17 25 10  VISUAL CLASSIFICATION   Mater content pearly equal to or greater than liquid limit.  The water content pearly equal to or greater than liquid limit.  The limit cates a non-plastic material with high water content.  Free water.  Auger boring plotted to vertical scale only.  B Indicates broken rock interval.	#	Elastic silt and clay with organic material	A-5 (9)	A÷5	0	3	28		25	46	NP	143	1		
Clay  A-7-6(12) A-7-6  I 1 37 52 13 18 29 7  VISUAL CLASSIFICATION  Auger boring - plan view.  Auger boring plotted to vertical scale only.  B Indicates broken rock interval.		Sift and clay	A-6 (8)	A-6a	13	5	- 11	33	38	30	12	19	215		
Couldery zone  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Shale  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  6  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  6  VISUAL CLASSIFICATION  Water content pearly equal to or greater than liquid limit.  O Indicates a non-plastic material with high water content.  We ree water.  Auger boring plotted to vertical scale only.  8  Indicates broken rock interval.	- Inches		A-6 (11)	A-6b	8	3	*7	38	xbi	38	17	25	10		
Shale  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Sandstone  VISUAL CLASSIFICATION  6  VISUAL CLASSIFICATION  6  VISUAL CLASSIFICATION  6  Water content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.  O mater content hearly equal to or greater than liquid limit.		Ctay	A-7-6(12)	A-7-6	6	1	14	37	52	,113	18	29	77		
Shale  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Water content pearly equal to or greater than liquid limit.  Water boring – plan view.  Auger boring plotted to vertical scale only.  Bern material.  Auger boring plotted to vertical scale only.  VISUAL CLASSIFICATION  Water content pearly equal to or greater than liquid limit.  Water content pearly equal to or greater than liquid limit.  Water content pearly equal to or greater than liquid limit.  Water content pearly equal to or greater than liquid limit.  Water content pearly equal to or greater than liquid limit.  Water boring plotted to vertical scale only.		Bouldery Zone			VISU	AL CLASSIFICA	TION								
Sandstone  Visual Classification  Visual Classification  Visual Classification  Visual Classification  Visual Classification  Visual Classification  Water content pearly equal to or greater than liquid limit.  Auger boring – plan view.  Auger boring plotted to vertical scale only.  Beautiful Classification  Visual Class	FT				VISUA	AL CLASSIFICA	ICATION								
Various other materials  VISUAL CLASSIFICATION  VISUAL CLASSIFICATION  Water content hearly equal to or greater than liquid limit.  Auger boring - plan view.  Auger boring plotted to vertical scale only.  Beautiful and classification  Water content hearly equal to or greater than liquid limit.  Indicates a non-plastic material with high water content.  Water content hearly equal to or greater than liquid limit.  Indicates a non-plastic material with high water content.  Water content hearly equal to or greater than liquid limit.  Indicates broken rock interval.		Shale			*VISU/	AL CLASSIFICA	TION						*8		
Sod and/or Topsoil=X'=Approximate depth.  Water content meanly equal to or greater than liquid limit.  How indicates a non-plastic material with high water content.  Water content meanly equal to or greater than liquid limit.  How indicates a non-plastic material with high water content.  Water content meanly equal to or greater than liquid limit.  How indicates a non-plastic material with high water content.  Water content meanly equal to or greater than liquid limit.					VISUA	AL CLASSIFICA	TION						6		
Sod and/or Topsoil_X'_Approximate depth.  Water content nearly equal to or greater than liquidalimit.  Hadger boring - plan view.  Auger boring plotted to vertical scale only.  Water content nearly equal to or greater than liquidalimit.  Hadger boring - plan view.  Ballotates broken rock interval.		Various other materials			VISUA	AL CLASSIFICAT	TION								
Berm material.  Auger boring - plan view.  Auger boring plotted to vertical scale only.  Berm material.  W Free water.  Buildicates a non-plastic material with high water content.  W Free water.  Buildicates broken rock interval.	-														
Auger boring - plan view.  B Indicates broken rock interval.		Sod and/or Topsoil=X'=Approximate de	epth.	1.9	₹, ₹		• Water	content pea	ly equal	to or greate	rothanoliquid	limit.			
Auger boring - plan view.  B Indicates broken rock interval.	XXX	Berm material.					O Indica	tes a non-p	lastic mat	erial with h	igh water con	tent.			
added bowing proceed to vertical scare only.	0	Auger boring - plan view.				-	₩ F	ree water.					,		
NOTE: Figures beside borings indicate water content in percent. e.g. /5	T	Auger boring plotted to vertical sca	ile only.				8 11	ndicates bro	ken rock	interval.					
NOTE: Figures beside borings indicate water content in percent. e.g. 15															
	NO1	E: Figures beside borings indicate water	er content in p	ercent. e	.g. 15										

GEAUGA - LAKE COUNTIES CEA - 44 - 10.00

TESTING LABORATORY

O. S. U. CAMPUS. COLUMBUS. OHIO

NOTE: INFORMATION SHOWN BY THIS SUBGRADE PROFILE WAS OBTAINED SOLELY FOR USE IN ESTABLISHING DESIGN CONTROLS FOR THE PROJECT. THE STATE OF OHIO DOES NOT GUARANTEE THE ACCURACY OF THIS DATA AND IT IS NOT TO BE CONSTRUED AS A PART OF THE PLANS GOVERNING CONSTRUCTION OF THE PROJECT.



LOCATION MAR

Recon - C.J.K., J.S.M. - 4/7/61 - 4/7/ Drilling - Auger - J.M., A.J.P., L.M.D., J.R.G. - 4/6/61-5/2/61 Drafting - R.C.B., R.A.W., C.L.Y. - 5/29/61

BOIL PROPILE
GEAUGA-LEAKE (COUNTIES
GEA-44-18.30
LAK-44=0.00
OHIO STATE HIGHWAY
TESTING LABORATORY
O. S. U. CAMPUS. COLUMBUS. OHIO

(Continued on following sheet.)

SUMMARY OF SCIL TEST DATA

NOTE: NP shown in tiquid timit and Plasticity Index columns indicates that the material is non-plastic.

\*Dénotes sample taken at or near grade.

	Denotes sample taken at or	near grade.	
STATION & OFFSET	STATION & OFFSET DEPTH S STATE SHITE	TATION & OFFSET	STATION & OFFSET CEPTH S L.L. P. II.
FRON-TO AGG. E.S. F.S. SILT CLAY W.C. CLASS.	FRON-TO AGG C.S. F.S. SILT QAY W.F. QASS.	FROM-TO AGG C.S. F.S. SILT Q AY W.C. Q ASS	FROM-TO AGG. C.S. F.S. SILT CLAY W.C. CLASS.
S.R. 44 (Westboun) Lanes)	S.R. 14 (Cont'd		S.R. 144 (Cont'd)
	501+00 CL 0.14-2.0 0 5 15 140 140 31 11 22 A-6a* 5	2+75 CL 0.3-1.5 4 8 16 37 35 24 6 24 A-14a 1.5-6.0 12 6 11 31 40 29 11 17 A-6a	593+50 CL 0.4-4.0 48 3 7 22 20 28 11 17 A-6a*
14+10 12'Lt 0.4-5.0 11 7 18 35 20 23 5 24 A-1ia* 5.0-10.0 13 11 17 30 29 24 8 14 A-1ia			508+00 CL 0.8-2.0 0 3 11 144 12 31 11 27 A-6a 2.0-4.0 15 14 10 33 38 29 11 18 A-6a
			598+00 CL 0.8-2.0 0 3 11 14 12 31 11 27 A-6a 2.0-4.0 15 4 10 33 38 29 11 18 A-6a 4.4a
17+58 21'Lt 0.4-5.0 0 7 14 38 41 25 11 13 A-6a 5.0-10.0 14 6 11 33 36 24 11 17 A-6a 10.0-12.0 8 6 11 34 41 25 11 16 A-6a	502+25 CL 0.4-5.0 8 9 10 39 34 28 11 17 A-6a 5 5.0-10.0 23 8 15 30 24 16 1 14 A-4a	ДН90 CL 0.0-6.0 27 6 12 25 30 31 12 19 A-ба 6.0-12.0 8 8 13 32 39 27 11 17 A-ба	
5.0-10.0 14 6 11 33 36 24 11 17 A-6a 10.0-12.0 8 6 11 34 41 25 11 16 A-6a			602+00 CL 0.3-4.0 8 7 14 35 36 25 11 15 A-6a*A 4.0-8.0 21 7 11 25 36 30 11 15 A-6a*A
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19+20 17'Lt 0.8-3.0 6 7 13 30 144 30 13 18 A-6a 3.0-6.0 13 11 28 20 28 15 3 17 A-14a 6.0-8.5 14 5 11 33 37 23 11 16 A-6a	3.0-6.0 23 6 10 27 34 29 11 14 A-6a 8.0-11.0 17 8 12 31 32 26 7 12 A-4a 11.0-16.0 16 10 16 28 30 23 8 10 A-4a		
	16.0-20.0 13 8 15 37 27 20 4 13 A-4a 1		606+00 10'Lt 0.3-5.0 9 5 87 39 39 31 11 21 A-6a*4 5.0-8.0 31 9 19 27 14 NP NP 15 A-4a 8.0-10.0 34 2 13 32 19 21 3 12 A-4a
22+27 14'Lt 0.4-1.0 12 7 28 35 20 NP NP 29 A-11a 1.0-3.0 50 6 18 15 11 NP NP 25 A-2-4	503+00 100'Rt 0.4-3.0 7 7 17 35 34 24 5 20 A-4a	17+00 CL 0.3-2.0 6 4 10 37 43 29 11 32 A-6a 2.0-5.0 10 7 15 34 34 29 12 24 A-6a	5.0-8.0 31 9 19 27 14 NP NP 15 A-4a 8.0-10.0 34 2 13 32 19 21 3 12 A-4a
23+75 42'Rt 0.4-2.0 0 2 14 52 32 33 9 35 A-1b 20-3.5 7 16 40 18 19 NP NP 28 A-1a	1 5 6-9 0 1 3 1 6 1 12 1 32 1 37 1 26 1 11 1 13 1 A-6am 1	19+50 CL 0,4-5,0 10 7 13 31 39 31 11 17 A-6a 5,0-10,0 12 9 13 30 30 30 11 12 A-6a 1	4.0-6.0   10   5   13   39   33   24   11   18   A-6a
23+75 (142'Rt 0.8-3.0 74 1 10 7 5 NP NP 21 A-1-a	9.0-14.0 15 6 12 28 39 26 11 13 A-6a* 14.0-17.0 7 8 13 31 41 26 7 15 A-1a	5.0-10.0   12   9   13   30   30   30   11   12   A-6a   15.0-20.0   9   6   13   32   10   28   11   13   A-6a	6,0-10.0 21 8 25 25 21 18 4 11 A-4a
	17.0-20.0 54 18 18 5 5 NP NP 1 9 A-1-a		609+50 75'Rt 10:4-4,0 28 5 13 28 28 30 11 29 A-6a+
5.R. 144	504+00 CL 0.4-3.0 13 4 14 29 40 40 14 25 A-6a 5	50+90 CL 0;3-2:0 12 4 11 31 42 32 13 21 A-6a 2;0-4:5 32 6 10 23 29 26 11 25 A-6a	611+00 CL 0.0-1.0 Sed ments 79 Visual * 1.0-4.0 19 6 14 30 31 29 11 22 A-6a *-
178+50 851Rt 0.4-1.0 Black Fine Textured Pett 250 Visual 1.0-4.0 6 6 18 142 28 36 8 149 A-14a			1,0-4,0 19 6 14 30 31 29 11 22 A-6a*- 4,0-10,0 0 1 89 - 10 - NP NP 22 A-3
	506+20 CL 0.4-5.0 18 8 12 30 32 21 6 15 A-4a 5 5 10.0-14.0 24 8 14 29 25 20 6 14 A-4a	55+00 CL 0.3-2.0 7 5 13 30 45 26 11 19 A-6a 2.0-5.5 20 6 12 29 33 27 11 15 A-6a	
179+50 CL 0.14-2.0 8 7 12 50 23 NP NP 25 A-116 14 36 14 30 11 26 A-6a	10.0-14.0 24 8 14 29 25 20 6 14 A-4a	57+00 751Rt 0.4-5.0 5 6 13 30 46 33 11 24 A-6a*	1 0.8-5.0 1 16 1 6 1 20 1 24 1 34 1 27 € 11 1 16 1 A-6a × 1
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15.0-8.0   8   7   12   33   140   31   12   13   A-5a	5.0-7.0 18 8 13 36 25 23 3 15 A-Ha 7.0-10.0 11 9 13 34 30 23 5 13 A-Ha		618+00 75'Rt 0.3-2.0 33 7 22 17 21 - NP NP 31 A-Ha * * 2.0-4.5 49 3 27 10 11 NP NP 20 A-1-b
8.0-13.0 11 6 6 39 38 28 11 13 A-6a 13.0-18.0 9 6 13 36 36 28 11 14 A-6a+ 18.0-20.0 7 6 14 35 38 28 11 14 A-6a+	509+76 75'Et 0.4-5,0 20 6 5 34 35 38 II 16 A-6a	\$2+00 75'Lt 0.0-3.0 13 2 10 34 41 34 12 30 A-6a 3.0-5.5 47 6 12 16 19 28 7 27 A-2-4	
13.0-18.0 9 8 13 36 38 28 11 14 14 14 14 14			620+00 68'Lt 0.0-2.0 Sed ments 158 Visual 2.0-4.0 39 2 28 18 13 NP NP 26 A-2-4
182+60 a. 0,4-3.0 8 6 14 31 41 32 11 18 A-64	510+00 CL 0.4+1.0 35 13 20 20 12 NP NP 28 A-2-4 5	\$2+00 CL 0.4-5.0 7 4 9 40 40 37 14 24 A-6a 5.0-7.0 28 21 25 9 17 29 11 17 A-2-5 7.0-12.0 32 10 16 24 18 21 4 16 A-4a	
	510+25 75'Rt 0.4-2.0 31 10 12 25 22 31 9 24 A-4a	7.0-12.0 32 10 16 24 18 21 4 16 A-4a 12.0-17.0 41 11 10 17 21 26 9 13 A-4a	621+25 75'Lt 0.0-1.0 Black clayey silt with plant fibers Visual 1.0-2.4 43 1 40 4 12 NP NP 25 A-2-4
11.0-10.0 9 6 13 36 37 29 7 14 A-Ha		12.0-17.0 11 11 10 17 21 28 9 13 A-14a 17.0-20.0 14 17 18 32 28 24 7 13 A-14a	
	511+95 CL 0.3-2.0 9 4 13 39 36 37 16 11 A-6b 2.0-4.0 20 6 13 33 28 27 11 17 A-6a A-6a 4.0-6 5.5-6.5 29 7 12 27 25 22 7 15 A-4a A-4a A-4a	52+00 75'Rt 0.4-4.0 0 1 8 144 19 112 21 35 A-7-6 3 33 63 144 18 36 A-7-6	1 0.8-4.0   11   6   15   32   36   27   11   21   A-6a**
1483+145 CL 0.14-5.0 13 6 11 27 143 37 13 23 A-6a 5.0-7.0 14 7 13 28 38 29 11 16 A-6a	2.0-4.0 20 6 13 33 28 27 11 17 A-6a 5.0-6.5 23 7 13 28 29 24 6 16 A-4a 5.6-6.5 29 7 12 27 26 22 7 15 A-1a	14.0-6.0 0 i 3 33 63 44 18 38 A-7-6	14.0-5.0 0 1 8 24 67 36 13 22 A-6a 5.0-6.0 53 2 10 14 21 31 12 24 A-6a
7.0-9.0   19   15   18   29   19   NP   NP   13   A-144		54+00 75'Lt 0.4-5.0 0 1 3 37 59 41 17 26 A-7-6 5.0-6.5 33 8 18 20 21 24 10 25 A-14a	
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	514+50 CL 0.4-4.0 16 9 15 29 31 29 11 22 A-6a 4-4a 9.0-13.0 13 7 13 29 38 25 7 14 A-4a A-4a 65	8,0-8,5 33 8 18 20 21 24 10 28 A-4a	623+00 80'Et 0.0-2.0 Sed ments 54 21 16 NP NP 34 A-4a
17.0-20.0 0 1 2 36 61 34 12 22 (A-64+	9.0-13.0 13 7 13 29 38 22 11 12 A-6a* 5	54+00 15'Lt 0.0-5.0 16 5 8 35 36 35 12 20 A-6a 5:0-9.0 14 9 15 35 26 28 7 15 A-1a	623+00 CL 0.8-2.5 9 4 31 28 30 22 5 21 A-Ha
1485+35 CL 0.4-3.0 3 5 12 35 145 140 18 20 A-66 3.0-7.0 10 7 12 31 140 33 13 18 A-6a	13.0-15.0 22 7 12 25 34 24 11 15 A-6a 15.0-20.0 11 7 14 31 37 22 11 14 A-6a	9.0-14.0   13   10   15   33   29   25   7   12   A-Ha	625100 CL 0.8-3.0 0 4 15 39 42 31 11 26 A-6a
3.0-7.0 10 7 12 31 140 33 13 18 A-6a 7.0-10.0 10 7 15 35 33 25 11 17 A-6a		14.0-20.0 15 11 13 33 28 NP NP 14 A-14	3:0-4:5 47 2 10 15 26 32 12 19 A-6a
7.0-10.0 10 7 15 35 33 25 11 17 A-6a 10.0-12.0 0 1 1 34 64 36 15 26 A-6a 12.0-17.0 11 12 20 34 23 23 7 16 A-4a 17.0-20.0 11 8 18 35 28 26 7 16 A-4a 17.0-20.0	516+90 CL 0.4-5.0 9 5 13 33 10 31 11 22 A-6a xx A-6a 10.0-12.0 22 5 12 29 32 27 11 12 A-6a 5	54+00 75'Rt 0.4-5.5 0 1 2 42 55 39 16 40 A-6b 5.5-6.5 0 1 4 45 50 28 11 25 A-6a	625+00 1001Rt 0.0-3.0 9 4 13 38 36 31 12 21 A-6a 5 11 33 42 27 11 17 A-6a
7.0-10.0 10 7 15 35 33 25 11 17 A-6a 10.0-12.0 0 1 1 34 64 36 15 26 A-6a 12.0-17.0 11 12 20 34 23 23 7 16 A-14a 17.0-20.0 11 8 18 35 28 26 7 16 A-14a 17.0-20.0		54+00 75'Rt 0.4-5.5 0 1 2 12 58 39 16 40 A-6b 5.5-6.5 0 1 4 15 50 28 11 25 A-6a 6.5-8.0 50 9 17 13 11 NP NP 15 A-1+b	
	517+50 CL 0.4-3,0 25 8 19 19 29 31 11 23 A-6a 3.0-0.0 33 6 15 27 19 21 5 12 A-4a 8.0-12.0 Gray Broken Shale 7 Vişûal	56+00 75'Lt 1.5-5.7 8 6 19 38 29 NP NP 24 A-14a	627+50 75!Lt 0.0-1.0 Sediments 77 Visual 1.0-4.0 39 5 18 22 16 24 6 23 A-4a
1486+63 CL 0.4-4.0 11 6 13 34 36 34 11 23 A-6a 14.0-7.0 0 1 3 33 63 45 16 26 A-7-6 7.0-12.0 12 8 16 32 32 29 11 15 A-6a-4-	8.0-12.0 Gray Broken Shale 7 Vişual		그는 사람들은 전혀 되는 사람들은 사실하게 하는 것이 되었다. 그는 사람들은 바람들은 사람들은 사람들이 되었다.
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187+60 75'[t 0.14-3.0 5 5 32 37 21 NP NP 23 A-14a	518+65 CL 0.4-2.0 40 6 15 16 23 31 9 27 A-4a 2.0-7.0 23 6 18 33 20 NP NP 17 A-4a 7.0-10.5 42 3 7 28 20 26 11 16 A-6a	8,0-13:0 12 9 15 30 34 23 11 11 A-64	
O. O. O. O. O. O. M. D. O. M. D. D. M. D.		56+00 75'Rt 0.3-4.0 0 2 3 50 45 34 9 29 A-46 4.0-6.5 42 9 11 21 17 26 9 22 A-4a	630+00 CL 1.0-5.0 6 5 12 36 41 31 11 25 A-6a <del>1</del> 5.0-7.0 30 7 25 26 12 NP NP 16 A-4a 7.0-10.0 10 3 9 50 28 NP NP 14 A-4b
1487+80 751Rt 0,4-2,0 0 3 28 44 25 46 NP 43 A-5	518+75 75'Rt 0.4-2.0 33 12 19 5 31 32 7 28 A-Ha 2.0-5.0 32 8 18 26 16 22 1 23 A-Ha 5.0-9.5 22 8 21 29 20 NP NP 10 A-Ha	4.0-6.5 42 9 11 21 17 20 9 22 A-18A	
191+00 100'Lt 0.4-2.0 Brown Broken Sandstone 2 23 Visual	5.0-9.5 22 8 21 29 20 NP NP 10 A-4a 5	59+00 CL 0,4-2.5 10 7 16 33 34 33 11 32 A-6a 2,5-3,8 13 5 13 26 43 32 11 23 A-6a	634+00 CL 1:0-6:0 18 6 14 28 34 28 11 26 A-6a*
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193+50 110'Lt 0.14-5.0 9 6 12 37 38 31 11 20 A-6a	7.0-11.0 13 6 15 33 33 24 7 14 A-4a	72+00 CL 0,4-3,0 5 4 13 32 46 36 15 21 A-6a	634+00 401Rt 0.8-5.0 17 9 14 30 30 31 11 22 A-6a ** 5.0-8.0 32 7 12 28 21 22 4 17 A-4a ** 8.0-14.0 31 8 8 28 25 22 5 12 A-4a
■ 1 5.0-10.0 10 7 13 35 35 26 11 14 A-6a	11.0-15.0 19 6 14 29 32 25 11 14 A-6a 15.0-17.0 10 4 9 27 50 29 11 17 A-6a	76+00 CL 0.4-2.0 3 4 11 35 47 37 12 25 A-6a 2.0-7.0 10 7 13 28 42 29 11 14 A-6a 7.0-12.0 8 6 11 31 44 30 11 14 A-6a 12.0-15.0 8 7 13 32 40 27 11 14 A-6a 15.0-20.0 14 10 18 38 20 NP NP 14 A-4a	
10.0-14.0 13 7 11 35 34 26 11 15 A-Sa 14.0-14.5 8 20 33 18 21 19 7 14 A-Ha		76+00 CL 0.4-2.0 3 4 11 35 47 37 12 25 A-6a 2.0-7.0 10 7 13 28 12 29 11 14 A-6a 7.0-12.0 8 6 11 31 44 30 11 14 A-6a 7.0-12.0 8 7 13 32 10 27 11 14 A-6a 7.0-12.0 11 10 18 38 20 NP NP 14 A-4a	635+25 20'Lt 0.8-5.5 19 5 12 33 31 30 11 17 A-6a*. 5.5-8.0 13 9 16 30 32 23 6 21 A-4a*. 8.0-13.0 21 9 11 32 27 21 4 13 A-4a. 13.0-16.0 45 5 16 15 19 20 4 9 A-2-4
193+50 50'Et 0.14-5.0 II 6 13 34 38 30 12 19 A-6a	523+00 CL 053-3,5 47 4 9 18 22 32 11 29 A-6a	7.0-12.0 8 6 11 31 44 30 11 14 A-6a# 12.0-15.0 8 7 13 32 40 27 11 14 A-6a# 15.0-20.0 14 10 18 38 20 NP NP 14 A-4a	5.5-8.0 13 9 16 30 32 23 6 21 A-la ** 8.0-13.0 21 9 11 32 27 21 14 13 A-la 13.0-16.0 145 5 16 15 19 20 14 9 A-2-14
5.0-10.0 8 7 13 39 33 28 11 14 A-54	526+50 CL 0.4-4.0 7 4 15 41 33 27 8 24 A-4a 4.0-6.0 17 5 11 30 37 32 11 19 A-6a 5		
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5.0-9.0 16 7 12 33 32 27 11 14 A-6a	535+00 75 Rt 1.0-2.0 26 6 11 28 29 26 11 20 A-6a 2.0-3.0 15 9 15 32 29 24 7 21 A-1a	AND THE TAX TO THE TAX TO TAX	650.00 (O)
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	537+70 CL 0.4-5,0 9 7 14 34 36 34 14 27 A-6a * 5.0-10.0 8 8 13 36 35 30 11 16 A-6a 5 10.0-15.0 13 .9 13 32 33 25 7 13 A-4a	88+00 CL 0.14-2.0 3 5 12 38 12 34 12 21 A-Ga	2.0-7.0 11 10 13 35 31 28 11 13 A-6a 7.0-11.0 11 10 12 38 29 27 7 13 A-4a 11.0-15.0 15 11 11 36 27 23 4 6 A-4a
5.0-8.0 8 7 14 34 37 28 11 14 A-Ca	5.0-10.0 8 8 13 36 36 30 11 16 A-6a 10.0-15.0 13 9 13 32 33 25 7 13 A-11a	2,0-7.0 13 8 14 30 35 33 11 18 A-6a *	grander and the second of the control of the contro
8.0-13.0 12 8 13 31 36 25 11 14 A-6a# 13.0-16.0 13 7 12 31 37 23 11 14 A-6a#	540+00 CL 0.4-2,0 3 4 11 31 51 40 15 24 A-6a	7.0-12.0 45 18 15 13 9 NP NP 9 A-1-b* 12.0-16.0 38 21 18 12 11 NP NP 9 A-1-b	653+90 CL 0.0-5.0 22 6 13 32 27 26 8 12 A-14a # 5.0-9.0 18 7 14 36 25 22 4 16 A-14a + 8.0-9.5 15 11 17 40 17 NP NP 11 A-14a 9:5-12:0 Gray Broken Sandstone 17 VIsual
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			a transfer and the contract of

SUMMARY OF SCIL TEST DATA (Cont'd)

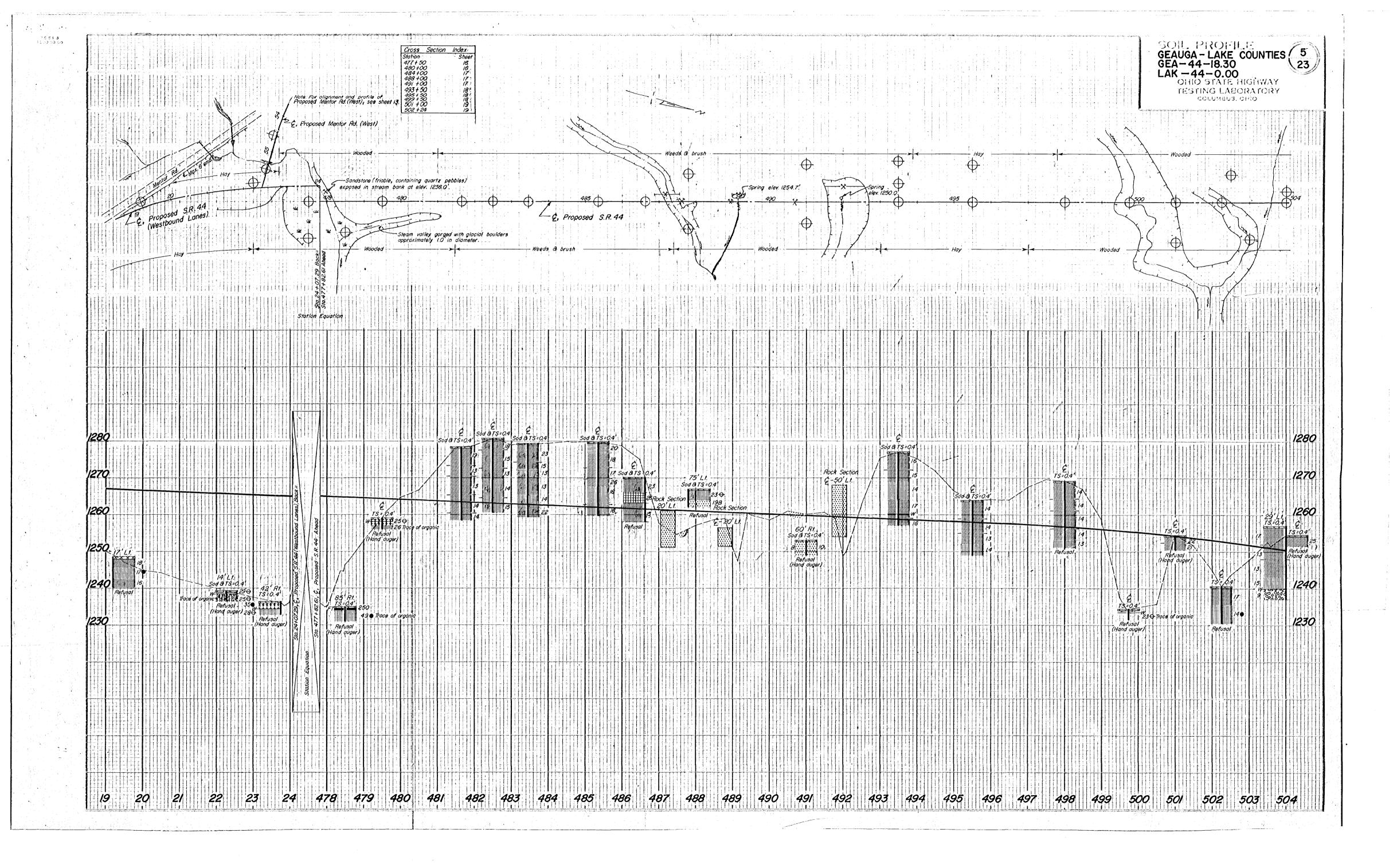
NOTE: NP shown in Liquid Limit and Plasticity Index columns indicates that the maternal is non-plastic.

\*Denotes sample taken at or near grade.

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	STATION	& OFFSET	DEPTH FROM+TO	AGG	*	%	*	*·	L.L.	P. I'.	*	SHTC		STATIO	N'& OFFSET	DEPTH	*	8	%	3 9	-: <b>1</b> 1	.L. P.11	*	SHTL	1	STATION	& OFFSET	CEPTH	7%	%	3	5	8 1	.L. P.	- T	SHTC	1
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ц	+00	251 Rt	0.3-1.5	0		14 14	38 64	41	29	11	27	'A-6a	•	57+00	140'Rt	0.4-3.0	7	a		14 (Cont)		,   ,	31	162 4		14+50	12'Rt	0.0-4.5	12	8	12	- 1	38	28	11 14	A-6a *	
	:		1.5-5.0 5.0-8.0 8.0-10.0	71	5 8	14 28 Sand	16	17	25.08 25.08 25.08	NP	15	A-6a *				3.0-7.0 7.0-10.0	12 8	74	9 4	38 4 39 4 37 4	5 3	2   11 7   11 19   17	24 25 27 21	A-6a ¥ A-6a € A-6b				4.5-8.0 8.0-10.0	18	6 7	13	888 88	38 36 41	28   26   25	11   12	A–€a A–6a	
8	+10	CL	0.3-2.0			1		12	NP	NP .	16	Visual A-3a		60+00	CL.	10.0-11:0	45	5	4	55 5	1 2	9 17		A-lia		18+50	50'Et	0.4-1.5	3 20	3 5	1 :: 1	29 25	54 39		3 23 15 18	A-6a	
			2.0-5.0 5.0-8.5	38 Gra	2 Broke	53 2 Shal	15 25	12 33	: NP : 35	15	15	A-6a Visual			<b>4</b>	0.4-2.0 2.0-6.0 6.0-7.0	Gray B	2	3 Shall	112 14 14	2 3	5 11	22   17   7	A-6a ≠ A-6a Visual		18+50	12184	0.0-6.5	19	7	13		1	- 1	11 12	A-60)A	-
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<b>      '</b>	5+00	15'Lt .	0.2-1.5 1.5-4.0 4.C-6.0	903	8	13   11   3	31 36 23	37 % 28	888	14 11 13	27 16 15	A-6a ** A-6a */* A-6a		63+00	140'Rt	0.4-3.0 3.0-6.0	0	2	3 5	42 5 5	3 4	5 11	25 23 15	A-6a A-7-6		26+50	12/Rt	6.5-7.5	19	7	12	i i	ı		٠, ا	'A-6a	
19	9+00	CL.	0.3-4.0	52	11	12	11	21	27	10	18	A-2-4		65+50	40 Rt	0.4-3.0	Gray B		Shall					Visual		20.00	12/11	4.5-7.0 7.0-10.0	13	77	12	37 32 29	143 36 37	27   1	6 20 1 16 6 12	A-6a	
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