	<b>STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS</b> REFER TO THE FOLLOWING STANDARD BRIDGE DRAWING(S):	<b>ROCK-SOCKETED SHAFTS</b> THE MAXIMUM FACTORED LOAI
	AS-1-15 DATED (REVISED) 1/20/23	SHAFT IS 278 KIPS AT THE ABUT
	AS-2-15 DATED (REVISED) 7/21/23	BY TIP RESISTANCE. AT THE REA
	PSID-1-13 DATED (REVISED) 1/20/23	IS 8,129 KIPS. AT THE FORWARD
	SBR-1-20 DATED (REVISED) 7/21/23	IS 11,113 KIPS.
	SICD-2-14 DATED (REVISED) 1/15/21	
	<b>DESIGN SPECIFICATIONS</b> THIS STRUCTURE CONFORMS TO THE 9th EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2020 AND THE ODOT BRIDGE DESIGN MANUAL, 2020.	DECK PLACEMENT DESIGN ASSU THE FOLLOWING ASSUMPTIONS METHODS WERE MADE FOR TH SUPERSTRUCTURE. THE CONTRA OF THE FALSEWORK SUPPORT S WILL ASSUME RESPONSIBILITY F
		DEVIATION FROM THESE DESIGI
	OPERATIONAL IMPORTANCE	
	A LOAD MODIFIER OF 1.0 HAS BEEN ASSUMED FOR THE DESIGN OF THIS	AN EIGHT WHEEL FINISHING MA
	STRUCTURE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 1.3.5 AND THE ODOT BRIDGE DESIGN MANUAL.	OF 2.46 KIPS.
		A MINIMUM OUT-TO-OUT WHE
	DESIGN LOADING	MACHINE OF 103".
	DESIGN LOADING INCLUDES:	
	VEHICULAR LIVE LOAD: HL-93	A MAXIMUM SPACING OF OVER
	FUTURE WEARING SURFACE (FWS) OF 0.060 KIPS/SF	
		A MAXIMUM DISTANCE FROM 1
	DESIGN DATA	TO THE FACE OF THE SAFETY HA
	CONCRETE CLASS QC2 - COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE)	
		TIEM 202, STRUCTURE REMOVI
	CONCRETE CLASS QC1 - COMPRESSIVE STRENGTH 4.0 KSI (SUBSTRUCTURE)	THIS ITEM SHALL INCLUDE REM ACCORDANCE WITH ITEM 202 V
	CONCRETE CLASS QC5, WITH 3/8" MAX. AGGREGATE SIZE - COMPRESSIVE	MAINTENANCE OF TRAFFIC. TH
	STRENGTH 4.5 KSI (DRILLED SHAFT)	BALLS WILL NOT BE PERMITTED
		PERMITTED WITHIN 3'-0" OF TH
	CONCRETE REINFORCEMENT:	REMOVAL. THE USE OF HOE-RAI
	EPOXY COATED STEEL REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI	REMOVAL. SUBMIT CONSTRUCT
		REINIOVAL. SUBIVITI CONSTRUCT
	(DECK, ABUTMENTS, DIAPHRAGMS, APPROACH SLAB)	
		THE EXISTING ABUTMENT CAPS
	GFRP REINFORCEMENT (BRIDGE RAILING)	EXISTING ABUTMENT COLUMNS
	STRUCTURAL STEEL - ASTM A709 GRADE 50 - YIELD STRENGTH 50 KSI	EITEM 503, UNCLASSIFIED EXCAN
		E UNCLASSIFIED EXCAVATION SHA
	CONCRETE FOR PRESTRESSED BEAMS:	<b>C</b> THAT BACKFILL MATERIAL PLACE
	COMPRESSIVE STRENGTH (FINAL) - 7.5 KSI	703.17 MATERIAL PLACED IN 6 I
	COMPRESSIVE STRENGTH (RELEASE) - 6.5 KSI	
		ITEM 503, COFFERDAMS AND E
	WELDED WIRE FABRIC:	THE DESIGN SHOWN ON THE PL
ngb.	YIELD STRENGTH - 70 KSI	EXCAVATION IS ONE REPRESENT
01.d		CONSTRUCT THE PROJECT. THE
SN001	PRESTRESSING STRAND:	DESIGN SHOWN ON THE PLANS
508	AREA = 0.217 SQ.IN.	SUPPORT THE SIDES OF EXCAVA
28015	ULTIMATE STRENGTH = 270 KSI	DESIGN FOR TEMPORARY SUPPO
	INITIAL STRESS = 202.5 KSI (LOW RELAXATION STRANDS)	PROVIDE PLANS IN ACCORDANC
SFN		WILL PAY FOR THE TEMPORARY
15823	MONOLITHIC WEARING SURFACE	CONTRACT LUMP SUM PRICE FC
	MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES,	BRACING. THE DEPARTMENT W
r. De Deets	TO BE 1 INCH THICK.	FOR PROVIDING AN ALTERNATE
USEK: bga )8\Sheets\		
AM 0150	MAINTENANCE OF TRAFFIC	
	FOR MAINTENANCE OF TRAFFIC NOTES AND DETAILS, SEE ROADWAY PLANS	
SFN (SFN		
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DAD TO BE SUPPORTED BY EACH DRILLED JTMENTS. THIS LOAD IS RESISTED ENTIRELY EAR ABUTMENT, THE FACTORED TIP RESISTANCE RD ABUTMENT, THE FACTORED TIP RESISTANCE

## SSUMPTIONS

ONS OF CONSTRUCTION MEANS AND THE ANALYSIS AND DESIGN OF THE TRACTOR IS RESPONSIBLE FOR THE DESIGN T SYSTEM WITHIN THESE PARAMETERS AND TY FOR SUPERSTRUCTURE ANALYSIS FOR IGN ASSUMPTIONS.

MACHINE WITH A MAXIMUM WHEEL LOAD

HEEL SPACING AT EACH END OF THE

ERHANG FALSEWORK BRACKETS OF 48 IN.

*1 THE CENTERLINE OF THE FASCIA GIRDER 1ANDRAIL OF 65"* 

OVED, OVER 20 FOOT SPAN, AS PER PLAN MOVAL OF THE EXISTING STRUCTURE IN 2 WHEN NO LONGER NEEDED FOR THE USE OF EXPLOSIVES AND HEADACHE ED. THE USE OF HOE-RAMS WILL NOT BE THE FULL DEPTH CUT LINE DURING PHASE 1 RAMS IS NOT RESTRICTED DURING PHASE 2 ICTION PLANS ACCORDING TO C&MS 501.05.

*PS SHALL BE REMOVED ENTIRELY AND THE INS LEFT IN PLACE.* 

## CAVATION, AS PER PLAN

HALL BE IN ACCORDANCE WITH 503 EXCEPT CED BEHIND THE ABUTMENTS SHALL BE 6 INCH LIFTS AS PER C&MS 304.05.

D EXCAVATION BRACING, AS PER PLAN PLANS FOR TEMPORARY SUPPORT OF NTATIVE DESIGN THAT MAY BE USED TO HE CONTRACTOR MAY CONSTRUCT THE NS OR PREPARE AN ALTERNATE DESIGN TO VATIONS. IF CONSTRUCTING AN ALTERNATE PPORT OF EXCAVATION, PREPARE AND NCE WITH C&MS 501.05. THE DEPARTMENT RY SUPPORT OF EXCAVATION AT THE FOR COFFERDAMS AND EXCAVATION WILL NOT MAKE ADDITIONAL PAYMENT ATE DESIGN.

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<i>F.F.</i> -
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BREVIATIONS JT. - ABUTMENT PR. - APPROACH PRX. - APPROXIMATE - AVERAGE DAILY TRAFFIC TT - AVERAGE DAILY TRUCK TRAFFIC . - BEARING - BENCHMARK VN. - BETWEEN . - BOTTOM - CENTER-TO-CENTER CENTERLINE - CONSTRUCTION JOINT - CORRUGATED PLASTIC PIPE . - CLEAR S OR C&MS - CONSTRUCTION AND MATERIAL SPECIFICATIONS NST. - CONSTRUCTION CREEK - CUBIC FEET PER SECOND DIAMETER 410 /G. - DRAWING - EACH FACE LPS σ OR ELEV. - ELEVATION T BED. - EMBEDMENT -00322-CH PHEI NOTES - EQUAL - EXISTING . - EXPANSION CH - FORWARD ABUTMENT GENERAL RAN GEA - FAR FACE FLOWLINE - FOOT Β 0 Z D. - FORWARD UTH S - FUTURE WEARING SURFACE ш *RP - GLASS FIBER REINFORCED POLYMER* BRIDGI 'ER SOL ' - HIGHWATER JOINT LEFT FORWARD LEFT 20 - MECHANICAL CONNECTOR X. - MAXIMUM S - MIDWEST GUARDRAIL SYSTEM I. - MINIMUM - NEAR FACE CPP - NON-PERFORATED CORRUGATED POLYETHYLENE PIPE - NUMBER *NM - ORDINARY HIGH WATER MARK* - OUT-TO-OUT PP - PERFORATED CORRUGATED POLYETHYLENE PIPE - PREFORMED EXPANSION JOINT FILLER FLOW - REAR ABUTMENT RADIUS Q'D - REQUIRED - RIGHT . - SERIES . - SPACES SQUARE FEET . - STATION -N 2801508 . - STANDARD ESIGN AGENCY - TOP OF ROCK fishbeck D.S. - TOE OF SLOPE - TOE-TO-TOE . - TYPICAL .O. - UNLESS NOTED OTHERWISE VELOCITY - VERTICAL POINT OF INTERSECTION DESIGNER CHECKER VR - WELDED WIRE REINFORCEMENT BMG TLC REVIEWER JBD 04/17/24 ROJECT ID 115823

UBSET

HEET

TOTAL

TOTAL

S.03 28

P.33 69