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<u>ITEM</u>

A MINIMUM OF ONE LANE OF TRAFFIC IN EACH DIRECTION SHALL BE MAINTAINED AT ALL TIMES, EXCEPT FOR A PERIOD NOT TO EXCEED 180 CONSECUTIVE CALENDAR DAYS, WHEN THROUGH TRAFFIC MAY BE DETOURED AS SHOWN ON SHEET 9 . A DISINCENTIVE SHALL BE ASSESSED IN THE AMOUNT OF \$2042 PER DAY FOR EACH CALENDAR DAY THE ROADWAY REMAINS CLOSED TO TRAFFIC BEYOND THE SPECIFIED LIMIT. THE DETOURS SHALL BE ESABLISHED, MAINTAINED, AND SUBSEQUENTLY REMOVED BY THE STATE OF OHIO. SEE SHEET 9 FOR CONTRACTOR/ ODOT SIGN LIST.

THE FIRST DAY THE DETOUR IN IN EFFECT SHALL BE CONSIDERED THE STARTING DATEOF THE 180 DAY DETOUR CLOSURE LIMITATION. THE 180TH DAY OF THE OF THE 180 DAY DETOUR/CLOSURE LIMITATION SHALL BE CONSIDERED AS AN INTERIM COMPLETEION DATE. ON OR BEFORE THE 180TH DAY, THE ROADWAY SHALL BE OPEN TO THE SAFE AND CONVENIENT USE OF THE TRAVELING PUBLIC. IF THE ROADWAY IS NOT OPENED BY THIS INTERIIM COMPLETION DATE, DISINCENTIVES SHALL BE ASSESSED AS PER THE ABOVE SPECIFICATION.

ACCESS TO ADJACENT PROPERTY WITHIN THE WORK LIMITS SHALL BE MAINTAINED BY THE CONTRACTOR AT ALL TIMES AS PER 614.02(a).

NOTICE OF CLOSURE SIGNS (W20-H13) SHALL BE ERECTED BY THE CONTRACTOR PRIOR TO THE SCHEDULED ROAD OR RAMP CLOSURE IN ACCORDANCE WITH THE NOTICE OF CLOSURE TIME TABLE BELOW. [AT THE APPROVAL OF THE ENGINEER, PORTABLE CHANGEABLE MESSAGE SIGNS MAY BE USED IN LIEU OF THE STANDARD FLATSHEET SIGN FOR CLOSURE DURATIONS OF LESS THAN 1 WEEK.

THE SIGNS SHALL BE ERECTED ON THE RIGHT-HAND SIDE OF THE ROAD/RAMP FACING TRAFFIC. THEY SHALL BE PLACED SO AS NOT TO INTERFERE WITH THE VISIBILITY OF ANY OTHER TRAFFIC CONTROL SIGNS. ON ROADWAYS, THEY SHOULD BE ERECTED AT OR NEAR THE POINT OF CLOSURE. THE SIGNS MAY BE ERECTED ANYWHERE ON RAMPS AS LONG AS THEY ARE VISIBLE TO THE MOTORISTS USING THE RAMP. ON ENTRANCE RAMPS, THE SIGN SHALL BE ERECTED WELL IN ADVANCE OF THE MERGE AREA TO AVOID DISTRACTING MOTORISTS.

NOTICE OF CLOSURE SIGN TII	ME TABLE
TEM DURATION	<u>SIGN DISPLAYED</u>
OF CLOSURE	TO PUBLIC

< 12 HOURS

SWITCHBOARD NUMBER.

ROAD	>=2 WEEKS	14 CALENDAR DAYS PRIOR TO CLOSURE					
CLOSURE	>= 12 HOURS & < 2 WEEKS	7 CALENDAR DAYS PRIOR TO CLOSURE					

2 BUSINESS DAYS PRIOR

TO CLOSURE THE SIGN SHALL DISPLAY THE DATE OF THE CLOSURE IN MMM-DD FORMAT AND THE NUMBER OF DAYS OF THE CLOSURE, THE LAST LINE OF THE W20-H13 SIGN LISTS A PHONE NUMBER WHICH A MOTORIST MAY CALL FOR ADDITIONAL INFORMATION. THIS IS TO BE A SPECIFIC OFFICE WITHIN THE DISTRICT RATHER THAN THE GENERAL

> SR698 WILL BE CLOSED (DATE) FOR 180 DAYS INFO: 419-999-6803

> > W20-H13-60

THROUGH OUT THE DURATION OF THE PRIJECT, THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER IN WRITING OF ALL TRAFFIC RESTRICTIONS AND UPCOMING MAINTENANCE OF TRAFFIC CHANGES. THE CONTRACTOR SHALL ENSURE THE WRITTEN NOTIFICATOIN IS SUBMITTED IN A TIMELY MANNER TO ALLOW THE PROJECT ENGINEER TO MEET THE REQUIRED TIME FRAMES SET FORTH IN THE TABLE BELOW TO INFORM THE SPECIAL HAULING PERMITS SECTION (HAULING PERMITS @DOT. OHIO.GOV)
AND THE DISTRICT PUBLIC INFORMATION OFFICE (PIO). THIS
NOTIFICATION SHALL BE RECEIVED BY THE PROJECT ENGINEER
PRIOR TO THE PHYSICAL SETUP OF ANY APPLICABLE SIGNS OR MESSAGE BOARDS.

INFORMATION SHOULD INCLUDE, BUT IS NOT LIMITED TO, ALL CONSTRUCTION ACTIVITIES THAT IMPACT OR INTERFERE WITH TRAFFIC AND SHALL LIST THE SPECIFIC LOCATION, TYPE OF WORK, ROAD STATUS, DATE AND TIME OF RESTRICTION, DURATION OF RESTRICTION, NUMBER OF LANES MAINTAINED, NUMBER OF LANES CLOSED, MINIMUM VERTICAL CLEARANCE, MINIMUM WIDTH OF DRIVABLE PAVEMENT, DETOUR ROUTES, IF APPLICABLE, AND ANY OTHER INFORMATION REQUESTED BY THE PROJECT ENGINEER.

NOTICE OF CLOSURE SIGN TIME TABLE

<u>ITEM</u>	<u>DURATION</u> <u>OF CLOSURE</u>	<u>SIGN DISPLAYED</u> <u>TO PUBLIC</u>
	>=2 WEEKS TO CLOSURE	21 CALENDAR DAYS PRIOR
ROAD CLOSED	>= 12 HOURS & < 2 WEEKS	14 CALENDAR DAYS PRIOR TO CLOSURE
	< 12 HOURS TO CLOSURE	4 BUSINESS DAYS PRIOR
LANE CLOSURES	>= 2 WEEKS	14 CALENDAR DAYS PRIOR TO CLOSURE
& RE- STRICTIONS	< 2 WEEKS	5 BUSINESS DAYS PRIOR TO CLOSURE
START OF COI TRAFFIC PATT		14 CALENDAR DAYS PRIOR TO IMPLEMENTATION

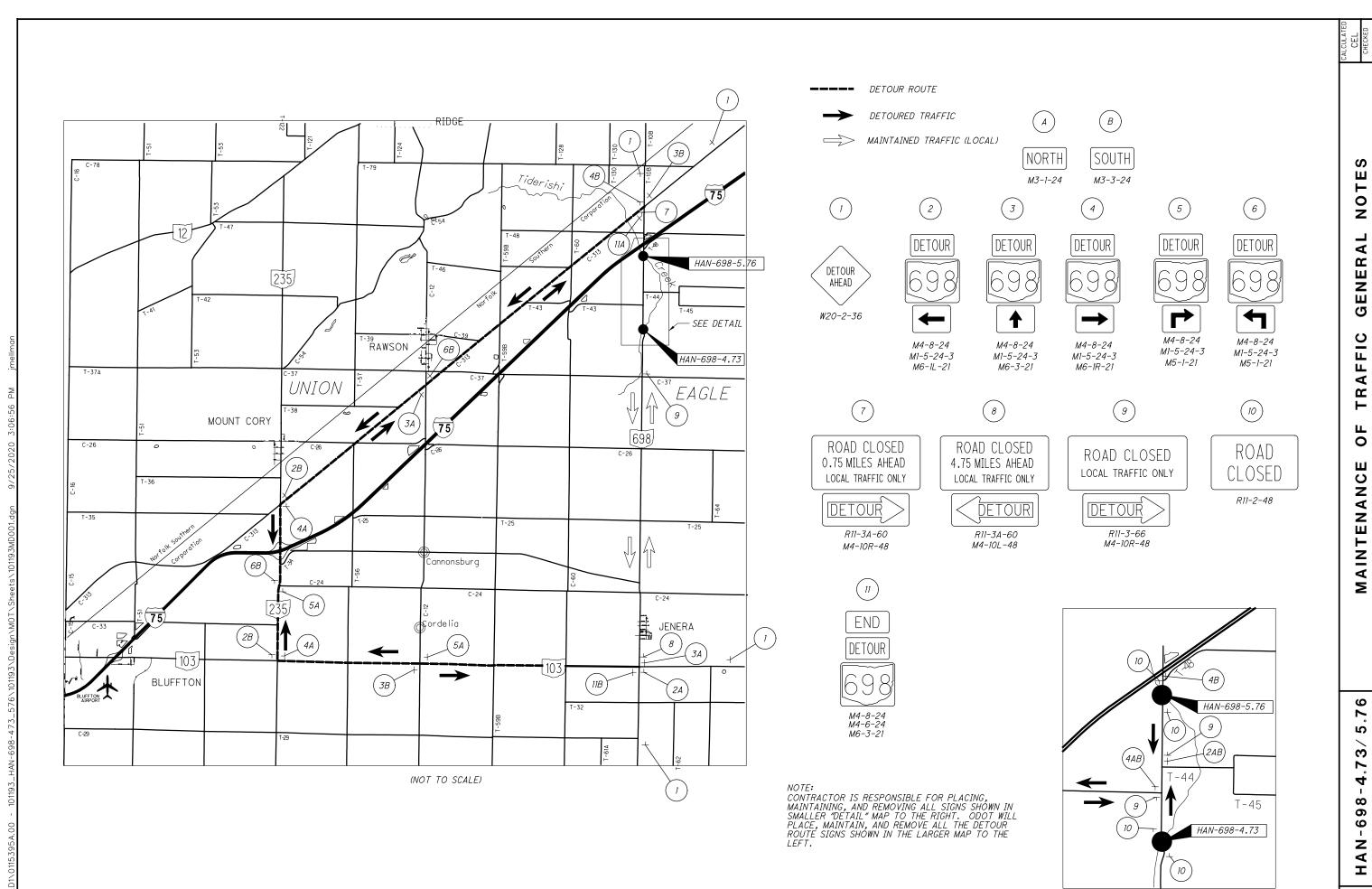
ANY UNFORESEEN CONDITIONS NOT SPECIFIED IN THE PLANS REQUIRING TRAFFIC RESTRICTIONS SHALL ALSO BE REPORTED TO THE PROJECT ENGINEER USING THE NOTIFICATION TIME TABLE.

ALL WORK AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH C&MS 614 AND OTHER APPLICABLE PORTIONS OF THE SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC. UNLESS SEPARATELY ITEMIZED IN THE PLAN.

DUST CONTROL

THE CONTRACTOR SHALL FURNISH AND APPLY WATER FOR DUST CONTROL AS DIRECTED BY THE ENGINEER. THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED FOR DUST CONTROL PURPOSES:

ITEM 616, WATER 1 M. GAL.



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DETAIL

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4 630 86002 4 EACH REMOVAL OF GROUND MOUNTED POST SUPPORT AND DISPOSAL 0.32 642 00104 0.32 MILE EDGE LINE, 6", TYPE I		
0.32 0.32 0.32 MILE EDGE LINE, 6", TYPE I		
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STRUCTURE HAN-698-4.73 ESTIMATED QUANTITIES	30	3
STRUCTURE HAN-698-5.76 ESTIMATED QUANTITIES	48	4
		1
INCIDENTALS		-
LS 614 11000 LS MAINTAINING TRAFFIC LS 614 12420 LS DETOUR SIGNING		-
6 619 16010 6 MNTH FIELD OFFICE, TYPE B		
LS 623 10000 LS CONSTRUCTION LAYOUT STAKES AND SURVEYING		
LS 624 10000 LS MOBILIZATION		

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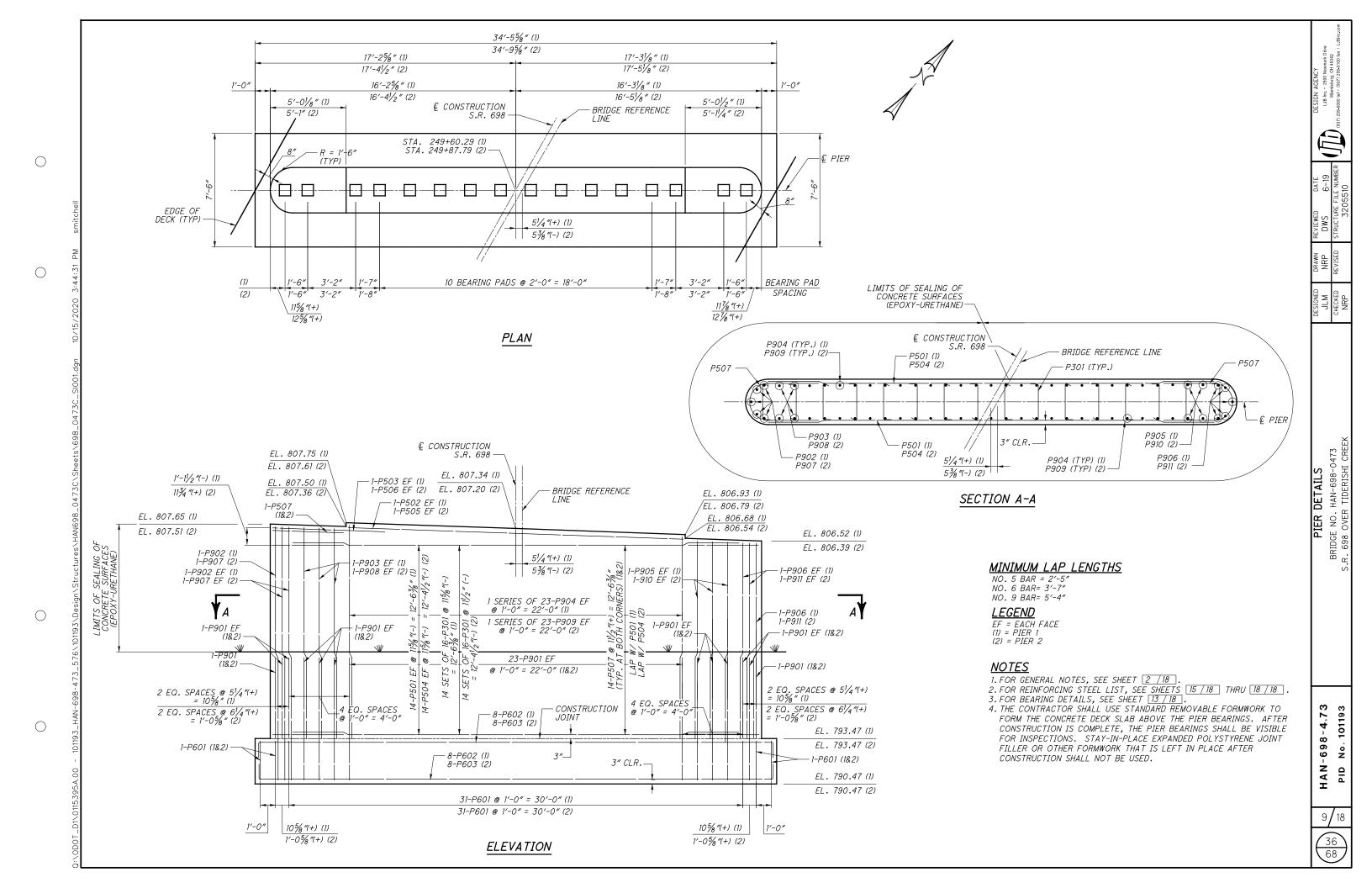
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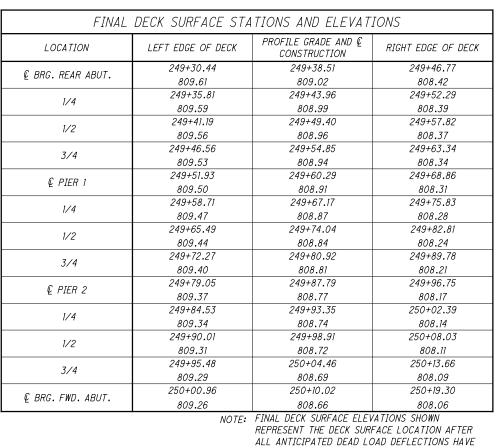
				ESTIMATED QUANTITIES					
ITEM	ITEM TOTAL UNIT DESCRIPTION 11002 LUMP STRUCTURE REMOVED, OVER 20 FOOT SPAN				ABUTMENTS	PIERS	SUPERSTRUCTURE	GENERAL	SHEET NO.
202	11002	LUMP		STRUCTURE REMOVED, OVER 20 FOOT SPAN				LUMP	
202	22900	132	SQ YD	APPROACH SLAB REMOVED				132	
503	11100	LUMP		COFFERDAMS AND EXCAVATION BRACING				LUMP	
503	21300	LUMP		UNCLASSIFIED EXCAVATION				LUMP	
503	31100	53	CU YD	ROCK EXCAVATION	13	40			
509	10000	67460	POUND	EPOXY COATED REINFORCING STEEL	32352	8121	26987		
511	32212	129	CU YD	CLASS QC2 CONCRETE WITH QC/QA, SUPERSTRUCTURE			129		
511	40512	96	CU YD	CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS		96			
511	43512	224	CU YD	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT INCLUDING FOOTING	224				
511	46512	58	CU YD	CLASS QC1 CONCRETE WITH QC/QA, FOOTING		58			
512	10100	218	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	42	122	54		
516	13900	113	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	113				
516	14020	92	FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL	92				
516	44001	70	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (9"X9"X1.6" BEARING WITH 10"X10"X1.5" LOAD PLATE)			70		13/18
517	70000	202	FT	RAILING (TWIN STEEL TUBE)			202		
518	21200	45	CU YD	POROUS BACKFILL WITH GEOTEXTILE FABRIC	45				
SPECIAL	51822300	171	FT	STEEL DRIP STRIP			171		
518	40000	120	FT	6" PERFORATED CORRUGATED PLASTIC PIPE	120				
518	40010	60	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	60				
526	25011	178	SQ YD	REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=15"), AS PER PLAN				178	14A/18, 14B/18
526	90010	68	FT	TYPE A INSTALLATION				68	
846	00110	28	CU FT	POLYMER MODIFIED ASPHALT EXPANSION JOINT SYSTEM				28	

QUANTITIES COMPUTED BY: NRP 03-19
QUANTITIES CHECKED BY: AMT 06-19

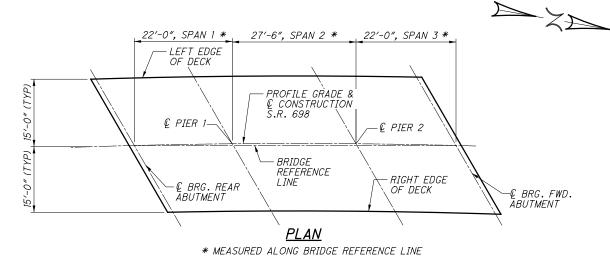
HAN-698-4,73

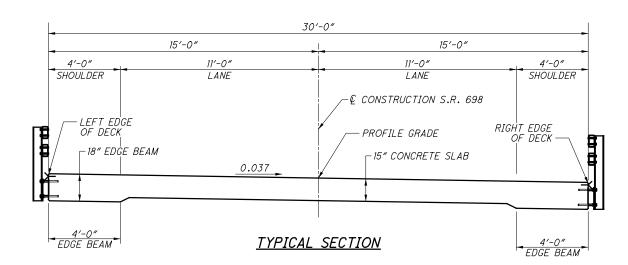
ESTIMATED QUANTITIES
BRIDGE NO. HAN-698-0473
S.R. 698 OVER TIDERISHI CREEK



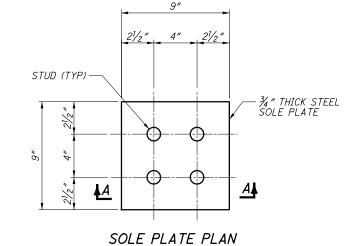


OCCURRED.





9"x9" LAMINATED 41/2" 41/2" ELASTOMERIC BEARING PAD € BEARING PAD AND ABUTMENT OR PIER **©** BEARING PAD PLAN



(BEARING PAD, LOAD PLATE AND

SLAB OR ABÚTMENT DIAPHRAGM CONCRETE, NOT SHOWN)

SECTION A-A

LAMINATED ELASTOMERIC BEARING DETAILS

€ BEARING PAD

3/4" \$\phi x5" END WELDED STUDS (TYP) -

3/4"x9"x9" STEEL SOLE PLATE,

SEE DETAIL ON THIS SHEET

11/2 "x10"x10" LOAD PLATE

LIVE LOAD REACTION (W/O IMPACT): 10.74 KIPS DEAD LOAD REACTION: 12.11 KIPS MAXIMUM DESIGN LOAD: 22.85 KIPS

PIER 1:

LIVE LOAD REACTION (W/O IMPACT): 21.09 KIPS DEAD LOAD REACTION: 18.24 KIPS MAXIMUM DESIGN LOAD: 39.33 KIPS

PIER 2:

LIVE LOAD REACTION (W/O IMPACT): 21.96 KIPS DEAD LOAD REACTION: 18.99 KIPS MAXIMUM DESIGN LOAD: 40.95 KIPS

BRG. = BEARING FWD. = FORWARD

LEGEND

NOTES

BOTTOM OF SLAB OR

ABUTMENT DIAPHRAGM

3-3/8" INTERIOR ELASTOMER LAYERS

3-14 GAGE INTERNAL STEEL LAMINATES

CONCRETE

TOP OF SEAT

1. FOR GENERAL NOTES, SEE SHEET 2 / 18

2. FOR APPROACH SLAB DETAILS, SEE SHEET 14/18 3. ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 50

- DUROMETER. THE BEARINGS WERE DESIGNED IN ACCORDANCE WITH SECTION 14.7.6 (METHOD A) OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THE LONG-TERM COMPRESSION PROOF LOAD TEST (AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, DIVISION II, SECTION 18.7.2.6) IS NOT REQUIRED.
- 4. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER THE BEARING IS INSTALLED.
- 5. THE STEEL LOAD PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 50 AND SHALL BE BONDED TO THE ELASTOMER BY VULCANIZATION DURING THE MOLDING PROCESS. TOP OF LOAD PLATES SHALL BE SHOP MARKED WITH PAINT INDICATING LOCATION ON THE BRIDGE AND FORWARD DIRECTION.
- 6. BASIS OF PAYMENT: THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS, LABOR AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL ELASTOMERIC BEARINGS. PAYMENT WILL BE AT THE CONTRACT PRICE FOR ITEM 516, EACH, ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE, AS PER PLAN, AS LISTED UNDER THE ESTIMATED QUANTITIES.
- 7. ALL STEEL LOAD PLATES AND STEEL SOLE PLATES OF THE ELASTOMERIC BEARINGS SHALL BE GALVANIZED ACCORDING TO CMS 711.02.

SUPERSTRUCTURE DETAIL
BRIDGE NO. HAN-698-0473
S.R. 698 OVER TIDERISHI CREE

101193 869 ° N HAN PID

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REAR APPROACH SLAB REINFORCING PLAN (TOP)

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SLEEPER SLABS AND GUARDRAIL SUPPORTS NOT SHOWN FOR CLARITY * MEASURED ALONG € CONSTRUCTION S.R. 698

FORWARD APPROACH SLAB REINFORCING PLAN (TOP)

SLEEPER SLABS AND GUARDRAIL SUPPORTS NOT SHOWN FOR CLARITY * MEASURED ALONG & CONSTRUCTION S.R. 698

	NUMBER		NUMBER		NUMBER		NUMBER		NUMBER		NUMBER		NUMBER		NUMBER			E						DIMFN	ISIONS					
MARK	REAR	FWD.	TOTAL	LENGTH	WEIGHT	TYPE		<u> </u>			1 _	I _		1 ,,				T												
							А	В	L	D	E	F	G	Н	K	0	R	INC												
					Α	<i>PPRO</i>	ACH SLA	BS (TOP)	- FOR I	NFORMA	TION ONL	Y																		
AS501	9		9	33′-6″	314	STR.																								
AS502	2		2	35′-4″	74	STR.																								
AS503	8		8	37′-1″	309	STR.																								
AS504	1	1	2	12'-4"	26	STR.																								
AS505	1		1	12′-9″	13	STR.																								
AS506	21	21	42	24'-7"	1077	STR.																								
AS507	1		1	10′-9″	11	STR.																								
AS508	1		1	11'-2"	12	STR.																								
AS509		8	8	38′-4″	320	STR.																								
AS510		1	1	36′-9″	38	STR.																								
AS511		1	1	36′-6″	38	STR.																								
AS512		9	9	35′-1″	329	STR.																								
AS513		1	1	11'-0"	11	STR.																								
AS514		1	1	10′-7″	11	STR.												<u> </u>												
AS515		1	1	12'-10"	13	STR.												L												

NOTE: THIS TABLE IS FOR INFORMATION ONLY. REINFORCING STEEL FOR APPROACH SLABS IS PAID FOR UNDER ITEM 526 - REINFORCED CONCRETE APPROACH SLAB WITH QC/QA (T=15"), AS PER PLAN.

<u>LEGEND</u>

EQ. = EQUAL SPA. = SPACING TYP = TYPICAL

BRG. = BEARING ABUT. = ABUTMENT FWD. = FORWARD

NOTES

- 1. FOR GENERAL NOTES, SEE SHEET 2 /18 . 2. FOR ADDITIONAL DETAILS, SEE STANDARD DRAWINGS AS-1-15 AND AS-2-15.
- 3. FIELD BEND OR FABRICATE BARS AS NECESSARY TO FIT RADIUS.
- 4. FIELD CUT BARS AS NEEDED TO ACHIEVE PROPER END COVER FROM EDGE OF APPROACH SLABS.

APPROACH SLAB REINFORCING (TOP)
BRIDGE NO. HAN-698-0473
S.R. 698 OVER TIDERISHI CREEK

101193

PID

HAN-698-4.73

REAR APPROACH SLAB REINFORCING PLAN (BOTTOM)

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SLEEPER SLABS AND GUARDRAIL SUPPORTS NOT SHOWN FOR CLARITY * MEASURED ALONG € CONSTRUCTION S.R. 698

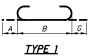
FORWARD APPROACH SLAB REINFORCING PLAN (BOTTOM)

SLEEPER SLABS AND GUARDRAIL SUPPORTS NOT SHOWN FOR CLARITY * MEASURED ALONG & CONSTRUCTION S.R. 698

	NUMBER					Ę						DIMEI	VSIONS					
MARK	REAR	FWD.	TOTAL	LENGTH	WEIGHT	TYPE			1	1	1	1	1	T	1	Г	T	
	ΛΕΑΛ	FWD.	TOTAL				А	В	С	D	Ε	F	G	Н	K	0	R	INC
	-	-		-	APF	PROAC	H SLABS	BOTTO	M) - FOR	INFORM	ATION O	NL Y		-	-	-	-	-
AS501	20		20	33′-6″	699	STR.												
AS502	3		3	35′-4″	111	STR.												
AS503	19		19	37′-1″	735	STR.												
AS509		19	19	38'-4"	760	STR.												
AS510		3	3	36′-9″	115	STR.												
AS512	1	21	22	35′-1″	805	STR.												
AS516	1		1	33′-9″	35	STR.												
AS517		1	1	38′-6″	40	STR.												
AS518		1	1	35′-10″	37	STR.												
AS1001	1		1	13′-9″	59	1	1′-5″	12'-4"										
AS1002	1		1	14'-0"	60	1	1′-5″	12'-7"										
AS1003	1		1	14'-3"	61	1	1′-5″	12'-10"										
AS1004	52	52	104	26′-0″	11635	1	1′-5″	24'-7"										
AS1005	1		1	12'-0"	52	1	1′-5″	10'-7"										
AS1006	1		1	12'-4"	53	1	1′-5″	10'-11"										
AS1007	1		1	12′-7″	54	1	1′-5″	11'-2"										
AS1008		1	1	12′-5″	53	1	1′-5″	11'-0"										
AS1009		1	1	12′-1″	52	1	1′-5″	10'-8"										
AS1010		1	1	11′-10″	51	1	1′-5″	10′-5″										
AS1011		1	1	14′-5″	62	1	1′-5″	13'-0"										
AS1012		1	1	14'-2"	61	1	1′-5″	12'-9"										
AS1013		1	1	13′-10″	60	1	1′-5″	12'-5"										

NOTE: THIS TABLE IS FOR INFORMATION ONLY. REINFORCING STEEL FOR APPROACH SLABS IS PAID FOR UNDER ITEM 526 - REINFORCED CONCRETE APPROACH SLAB WITH QC/QA (T=15"), AS PER PLAN.

BENDING DIAGRAMS



<u>LEGEND</u>

EQ. = EQUAL BRG. = BEARING SPA. = SPACING ABUT. = ABUTMENT TYP = TYPICAL FWD. = FORWARD

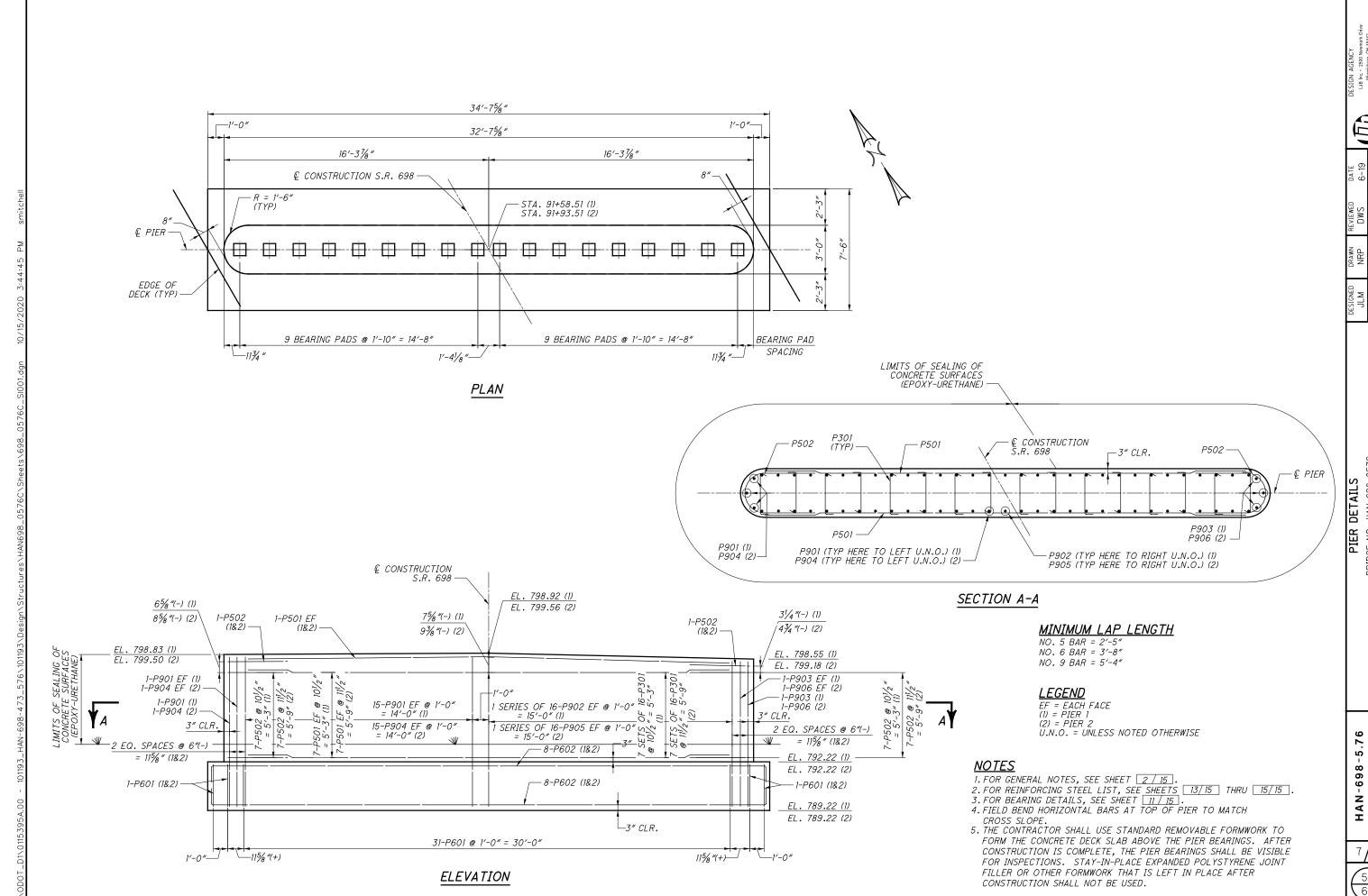
NOTES

- 1. FOR GENERAL NOTES, SEE SHEET 2 /18 . 2. FOR ADDITIONAL DETAILS, SEE STANDARD DRAWINGS
- AS-1-15 AND AS-2-15.
- 3. FIELD BEND OR FABRICATE BARS AS NECESSARY TO FIT RADIUS.
- 4. FIELD CUT BARS AS NEEDED TO ACHIEVE PROPER END COVER FROM EDGE OF APPROACH SLABS.

PID

SLAB REINFORCING (BOTTOM)
RIDGE NO. HAN-698-0473
698 OVER TIDERISHI CREEK APPROACH SLAB

BRIDGE N
S.R. 698 O' 101193 HAN-698-4.73



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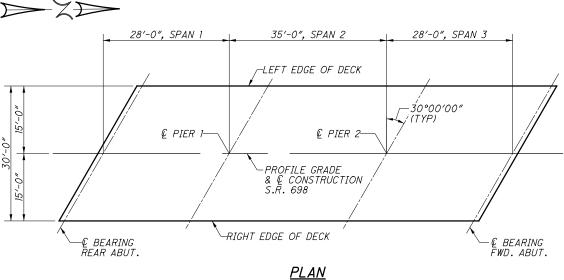
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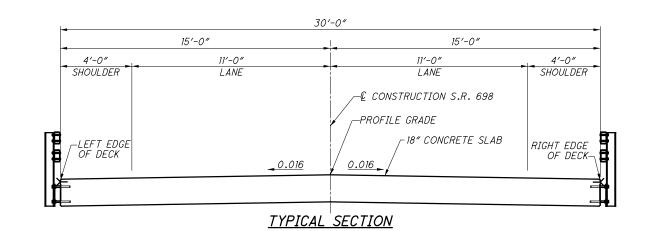
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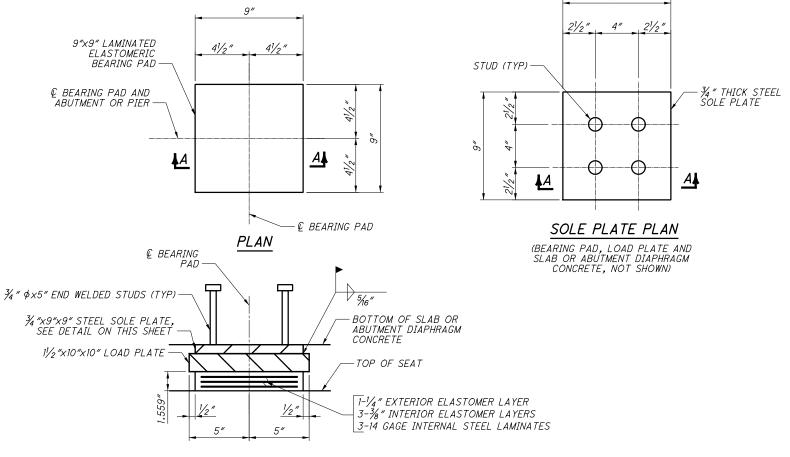
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LAMINATED ELASTOMERIC BEARING DETAILS

SECTION A-A

ABUTMENTS:

LIVE LOAD REACTION (W/O IMPACT): 10.36 KIPS DEAD LOAD REACTION: 13.00 KIPS MAXIMUM DESIGN LOAD: 23.36 KIPS

PIERS:

LIVE LOAD REACTION (W/O IMPACT): 16.56 KIPS DEAD LOAD REACTION: 18.73 KIPS MAXIMUM DESIGN LOAD: 35.29 KIPS

NOTES

- 1. FOR GENERAL NOTES, SEE SHEET 2 / 15
- 2. FOR APPROACH SLAB DETAILS, SEE SHEET 12/15.
- 3. ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED IN ACCORDANCE WITH SECTION 14.7.6 (METHOD A) OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THE LONG-TERM COMPRESSION PROOF LOAD TEST (AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, DIVISION II, SECTION 18.7.2.6) IS NOT REQUIRED.
- 4. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER THE BEARING IS INSTALLED.
- 5. THE STEEL LOAD PLATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM A709 GRADE 50 AND SHALL BE BONDED TO THE ELASTOMER BY VULCANIZATION DURING THE MOLDING PROCESS. TOP OF LOAD PLATES SHALL BE SHOP MARKED
- WITH PAINT INDICATING LOCATION ON THE BRIDGE AND FORWARD DIRECTION.
 6. BASIS OF PAYMENT: THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS, LABOR AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL ELASTOMERIC BEARINGS. PAYMENT WILL BE AT THE CONTRACT PRICE FOR ITEM 516, EACH, ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE, AS PER PLAN, AS LISTED UNDER THE ESTIMATED QUANTITIES.
- 7. ALL STEEL LOAD PLATES AND STEEL SOLE PLATES OF THE ELASTOMERIC BEARINGS SHALL BE GALVANIZED ACCORDING TO CMS 711.02.

SUPERSTRUCTURE DETAIL
BRIDGE NO. HAN-698-0576
S.R. 698 OVER TIDERISHI CREE

101193 HAN-698-5 ° N Δ