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Project LAK-20-1959

Project No. 18 0743 Sheet No. 1 of 4

Calculated By JPR Date 02/04/22

Checked By \_\_\_\_\_ Date \_\_\_\_\_

Subject Culvert STA. 313+72, Box Extension Walls Bars

C601 - Wall Dtls, MF, Vert

$B = 3'-10"$   
 $A = (638.24 - 632.37) = 5.87' + (12" \text{ Embed}) = 6.87' - (2 \frac{1}{2}" \text{ cl.}) = 6.66' \Rightarrow 6'-8"$   
 $B = 46" = 3'-10"$   
 $L = 6'-8" + 3'-10" = 13'-6" - (2") = 13'-4"$   
 No. Reqd = 2 walls x 17 = 34  
 Inlc. Area Amt @ 0.2131 =  $(0.2131)(12' + 1') = 40.7' \Rightarrow 41'$   
 $41' + \frac{1}{2}(13') - 2" = 45.5' \Rightarrow 46" = 3'-10" \leftarrow \text{use}$   
 Class C Splice #5  
 $= (1.7 \times 15') / (0.8)(1.20)(1.14) = 34.3"$

C601 Wall Dtls, F.F. Vert.

$L = (638.24 - 632.37) = 5.87' + (12" \text{ Embed}) = 6.87' - (2 \frac{1}{2}" \text{ cl.}) = 6.66' = 6'-8"$   
 No. Reqd = 2 walls x 9 = 18

C602 - Wall Dtls, MF, Vert

$A = (638.24 - 636.10) = 2.14' + (12" \text{ Embed}) = 3.14' - (2 \frac{1}{2}" \text{ cl.}) = 2.93' \Rightarrow 2'-11"$   
 $B = 3'-10"$  (see C601)  
 $L = 2'-11" + 3'-10" = 6'-9" - (2") = 6'-7"$   
 No. Reqd = 2 walls x 17 = 34

C602 Dtls, Wall Dtls: F.F. Vert + Centered Horiz. + Extra Diagonal E.F. And Roof Slab Centered Butt Jnt.

$L = (638.24 - 636.10) = 2.14' + (12" \text{ Embed}) = 3.14' - (2 \frac{1}{2}" \text{ cl.}) = 2.93' = 2'-11"$   
 No. Reqd = 2 walls x 9 = 18  
 + 2 walls x (1+1) = 4  
 + 2 walls x (2x1) = 4  
 + Roof Slab Butt Jnt = 11  
 $\underline{\quad\quad\quad 37}$

C501 - Walls, Horiz.

$L = 8'-6" - (3" + 3" \text{ cl.}) = 8'-0"$   
 No. Reqd = 2 walls x (2x3) = 12

C502 - Walls, Horiz. and Roof Slab, Longitudinal

$L = 17'-6" - (2 \frac{1}{2}" + 2 \frac{1}{2}" \text{ cl.}) = 17'-1"$   
 No. Reqd = 2 walls x (2x2) = 8  
 + (2x10) = 20  
 $\underline{\quad\quad\quad 28}$


CS03 - Roof Slab, Transverse Top  
 Width of Ex. Part Slab =  $(1'-1" + 12'-0" + 1'-1") = 14'-2"$   
 $L = 14'-2" - (2 \times 2\frac{1}{2}" \text{ clc}) = 13'-9"$   
 No. Reqd = 10

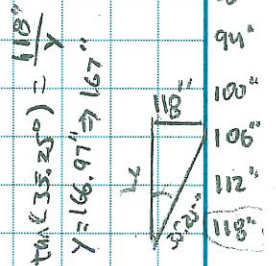
CS04 - Roof Slab, Transverse Top - Ser. of 14  
 Short Bar =  $57" - (2\frac{1}{2}" + 3\frac{1}{2}" \text{ clc}) = 51" = 4'-3"$   
 Long Bar =  $167" - (2\frac{1}{2}" + 3\frac{1}{2}" \text{ clc}) = 161" = 13'-5"$   
 Inc. =  $\frac{(161 - 51)"}{13 \text{ sps.}} = 8.46" \Rightarrow 8\frac{1}{2}"$   
 2 - Ser. of 14

CB03 - Roof Slab, Transverse Bot.  
 $L = 14'-2" - (2 \times 2\frac{1}{2}" \text{ clc}) = 13'-9"$   
 No. Reqd = 10

CB04 - Roof Slab, Transverse Bot - Ser. of 14  
 (Same Lengths as CS04 - Short Bar = 4'-3"  
 Long Bar = 13'-5" } Inc. = 8 1/2"  
 2 - Ser. of 14

CS05 - Roof Slab, Fanned Bot @ Two Acute Corners  
 (Same Length as Horiz. Leg of CB10 DWLS. = 3'-10"  
 No. Reqd = 2 Corners  $\times$  6 = 12

CS05 - Foreslope Wall Ends  

 $A = (6" + 1'-5") = 1'-11" - (2" + 3" \text{ clc}) = 1'-6"$   
 $B = 1'-3" - (2" + 2" \text{ clc}) = 11"$   
 $L = 1'-6" + 11" + 1'-6" = 3'-11" (11\frac{1}{2}) = 2'-10"$   
 No. Reqd =  $(14'-2" \div \cos(35.25^\circ)) = 17.35 \div 1.0 = 17.35 \text{ sps} \Rightarrow 18$



4"  
 10"  
 16"  
 22"  
 28"  
 34"  
 40"  
 46"  
 52"  
 58"  
 64"  
 70"  
 76"  
 82"  
 88"  
 94"  
 100"  
 106"  
 112"  
 118"



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Project LAK-20-19.59  
 Project No. 180743 Sheet No. 3 of 4  
 Calculated By JPR Date 6/9/22  
 Checked By \_\_\_\_\_ Date \_\_\_\_\_  
 Subject Culvert @ STA. 313+72 - Reinf. Bas

Additional steel Bars requested by DDOT (Stage 3 renewal, Comments # 448 & #450)

#448 - ... Need a edge beam. Add Top and Bottom rebar parallel to the skewed end of the roof slab, similar to the  
 sub B26 design shown in the existing plans  $\Rightarrow$  H8a =  $\#8 \times 17'-0"$  Long (STR)

#450 - Add Top and Bottom rebar parallel to the skewed end with the C505 stirrup shown in Section B-B  
 sub B37 to create an edge beam

C806 - Edge Beam, 4T & 4B

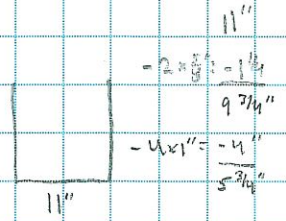
Out-out roof slab =  $(7'-1" + 7'-1") = 14'-2"$ , Along skewed =  $(14.17' \cos 35^\circ 15') = 11.35'$

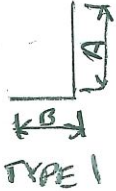
$L = 11.35' - (2 \times 42" / 12) = 10'-0"$

No. Reqd =  $2 \times 4 = 8"$

Check clearance in side C505

$$\frac{5.75'}{3 \text{ spcs}} = 1.92" \text{ gap} > 1" \text{ Min. reqd}$$





CONCRETE BOX CULVERT EXTENSION

MARK	NUMBER	TOTAL	LENGTH	WEIGHT	TYPE	DIMENSIONS					INC	
						A	B	C	D	E		
C501	12		8'-0"	100	STR.							
C502	28		17'-1"	499	STR.							
C503	10		13'-9"	143	STR.							
C504	2-SER. OF		4'-3"	258	STR.							
		14	TO									
			13'-5"									
C505	18		3'-10"	72	STR.	1'-6"	11"	1'-6"				8 1/2"
C601	18		6'-8"	180	STR.							
C602	37		2'-11"	162	STR.							
C808	8		17'-5"	303	STR.							
C801	34		13'-4"	1210	1	6'-8"	3'-10"					
C802	34		6'-7"	598	1	2'-11"	3'-10"					
C803	10		13'-9"	367	STR.							
C804	2-SER. OF		4'-3"	66	STR.							
		14	TO									
			13'-5"									
C805	12		3'-10"	123	STR.							

TOTAL = 4,301 LBS  
 11 STR 11-0-0

X601 - W/Wall, FF Vert.

$$L = 12'-5'' - (2'' \text{ (1x)}) = 12'-3''$$

$$\text{Spaces @ } 16'' : 22'-0'' \div 1.33' = 16.54 \Rightarrow 16 \text{ spa.} = 17 \text{ Bars ; No. = 2 walls } \times 17 = 34$$

Y601 - W.W., FF Vert. Out

$$B = 2'-0'' + 5'-2'' \text{ Lap} = 7'-2'' = (3'' \text{ (1x)}) = 6'-11''$$

$$R = 1'-0''$$

$$L = 14'-0'' + 6'-11'' = 20'-11'' - (2'') = 20'-9''$$

$$\text{Spaces @ } 8'' : 22'-0'' \div 0.67' = 32.8 \Rightarrow 32 \text{ spaces} = 33 \text{ Bars ; No. = 2 walls } \times 33 = 66$$

$$R = 1'-0''$$

WWS01 - W.W., NF Vert.

$$L = 12'-5'' - (2'' \text{ (1x)}) = 12'-3''$$

$$\text{Spaces @ } 18'' : 22'-0'' \div 1.5' = 14.7 \Rightarrow 14 \text{ spaces} = 15 \text{ Bars ; No. = 2 walls } \times 15 = 30$$

WWS02 - W.W., Horiz. EF

$$L = 22'-0'' - (2'' + 2'' \text{ (2x)}) = 21'-8''$$

$$\text{Spaces @ } 18'' : 12'-5'' \div 1.5' = 8.28 \Rightarrow \text{Spac.} = 9 \text{ Bars ; No. = 2 walls } \times (2 \times 9) = 36$$

V601 - Fty, Top Trans.

$$L = 9'-9'' - (3'' + 3'' \text{ (1x)}) = 9'-3''$$

$$\text{No. Req'd} = (24 + 5) + 29 = 58$$

V602 - Fty, Horiz. Dnls. Into Ex. Cut-off wall above and Fanned top, East Wall Fty

$$L = 4'-3'' + (1'-0'' \text{ embed}) = 5'-3'' - (3'' \text{ (1x)}) = 5'-0''$$

$$\text{No. Req'd} = 2 \text{ Rows} \times 23 = 46 + 4 = 50$$

W601 - Fty, Bob. Trans.

$$L = 9'-9'' - (3'' + 3'' \text{ (1x)}) = 9'-3''$$

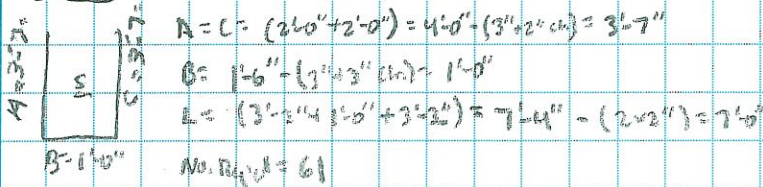
$$\text{No. Req'd} = (18 + 3) + 22 = 43$$

W602 - Fty, Horiz. Dnls. Into Ex. Cut-off wall Bob and Fanned Bob, East wall Fty

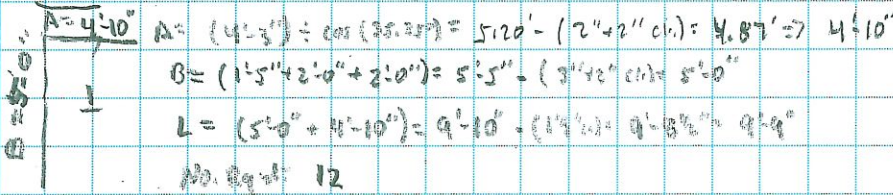
$$L = 4'-3'' + (1'-0'' \text{ Embed}) = 5'-3'' - (3'' \text{ (1x)}) = 5'-0''$$

$$\text{No. Req'd} = 17 + 2 = 19$$

F601 - Cut-off Wall, Vert.



F501 - Cut-off wall (vert) into Closure Slab



F502 - Closure Slab, Horiz

$L = 17'-4\frac{1}{2}" - (2' + 2") = 17'-0\frac{1}{2}" \Rightarrow 17'-0"$   
 No. Reqd = 2

F601 - Footing, T. 4B, Longitudinal

Cover  $6'-4\frac{1}{2}" - (3" + 3" \text{ cov.}) = 6'-10"$  with two Bars and  $\frac{2'-11"}{3'-5"}$  Lap  
 $6'-10" \times \frac{1}{2} = 3'-5"$  +  $\frac{1}{2} (\frac{3'-5"}{2'-11"}) = 3'-13" = \frac{3'-11"}{2'-11"}$   
 No. Reqd =  $2 \times (2 \times 4) = 16$

class C Spacing 6"  
 $(1.7 \times 18") (1.20) (0.8) (1.0) = 41" = 3'-5"$   
 Use 2-11" per OOT Plan sheet  
 sheet

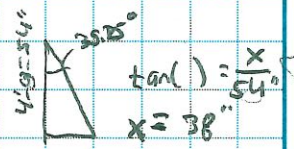
F602 - Cut-off Wall, Longitudinal

(Same Length as F601:  $\frac{3'-11"}{2'-11"}$ )  
 No. Reqd =  $2 \times 2 = 4$

F603 - West Wall Ftg., T. 4B, Longitudinal - Ser. of 4

Short L =  $17'-8\frac{1}{2}" - (3" + 3" \text{ cov.}) = 17'-2" = 206" = 17'-2"$   
 Long L =  $(206" + 38") = 244" \Rightarrow 242" = 20'-2"$   
 $2 \times 1 = 2$  - Ser. of 4

$\text{Incl} = \frac{36"}{3 \text{ sp}} = 12"$



F604 - East Wall Ftg., T. 4B, Longitudinal, - Ser. of 4

Short L =  $25'-6" - (3" + 3" \text{ cov.}) = 25'-0" = 300" = 25'-0"$   
 Long L =  $(300" + 38") = 338" \Rightarrow 336" = 28'-0"$   
 $2 \times 1 = 2$  - Ser. of 4

$\text{Incl} = \frac{36"}{3 \text{ sp}} = 12"$

F5501 - Closure Wall, Horiz EF

$L = (14'-2") \div \cos(35.25^\circ) = 17'-3" - (2" + 2" \text{ cov.}) = 17'-0"$  No. Reqd =  $(2 \times 1) = 2$



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Project LNK-20-14.59  
 Project No. 180140 Sheet No. 3 of 3  
 Calculated By JPR Date 2/11/22  
 Checked By \_\_\_\_\_ Date \_\_\_\_\_  
 Subject Headwall, Culvert STA 313+72, RT.

TYPE C HEADWALL REINFORCING SCHEDULE

BAR MARK	NUMBER	LENGTH	WEIGHT	TYPE	A	B	C	D	E	R	INCL
WING-WALLS											
XG01	34	12'-3"	626	STR.							
YG01	26	7'-9"	768	1	1'-0"	6'-11"					
WNWS01	30	12'-3"	383	STR.							
WNWS02	36	21'-8"	814	STR.							
FOOTINGS & CUTOFF WALL											
VG01	58	9'-3"	806	STR.							
VG02	50	5'-0"	376	STR.							
WG01	43	9'-3"	597	STR.							
WG02	19	5'-0"	143	STR.							
ZG01	5	7'-0"	53	5	3'-7"	1'-0"	3'-7"				
FJ01	12	9'-9"	122	1	4'-0"	5'-0"					
FJ02	2	17'-0"	35	STR.							
FG01	16	31'-11"	767	STR.							
FG02	4	31'-11"	192	STR.							
FB03	2-SER.	17'-2"	224	STR.							1'-0"
	OF	TO									
	4	20'-2"									
FG04	2-SER.	25'-6"	318	STR.							1'-0"
	OF	TO									
	4	28'-0"									
FORESLOPE WALL											
FS01	2	17'-0"	35	STR.							
											TOTAL = 6,259 LBS

3,591

3,687

35

**GENERAL NOTES**

**DESIGN SPECIFICATIONS:** THIS STANDARD DRAWING CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2002 AND THE ODOT BRIDGE DESIGN MANUAL.

**DESIGN DATA:** THE FOLLOWING DESIGN DATA IS ASSUMED:

- INTERNAL ANGLE OF FRICTION OF BACKFILL SOIL,  $\phi_{bf} = 30^\circ$
- TOTAL UNIT WEIGHT OF BACKFILL SOIL = 120 PCF
- INTERNAL ANGLE OF FRICTION (DRAINED), FOUNDATION SOIL,  $\phi_f = 30^\circ$
- UNDRAINED SHEAR STRENGTH (COHESIVE), FOUNDATION SOIL,  $S_{uf} = 1500$  PCF
- UNIT WEIGHT OF CONCRETE = 150 PCF
- SLOPE OF BACKFILL = 2:1 (TYPE A & B HEADWALLS)
- HEIGHT OF LIVE LOAD SURCHARGE = 2 FT (TYPE C HEADWALLS)

CONCRETE - COMPRESSIVE STRENGTH 4000 PSI (FOOTING, WINGWALL AND FORESLOPE WALL)

REINFORCING STEEL - ASTM A615, A616, OR A617  
GRADE 60 MINIMUM YIELD STRENGTH  
60,000 PSI (ALL REINFORCING SHALL BE EPOXY COATED)

BASED ON THE ASSUMED DESIGN DATA, THE WINGWALLS ACHIEVE FACTORED BEARING RESISTANCES THAT ARE GREATER THAN THEIR RESPECTIVE BEARING PRESSURES. IF A BACKFILL MATERIAL WITH A HIGHER INTERNAL ANGLE OF FRICTION OR A LIGHTER TOTAL UNIT WEIGHT IS USED; OR IF A FOUNDATION SOIL WITH A HIGHER DRAINED INTERNAL ANGLE OF FRICTION OR A HIGHER UNDRAINED SHEAR STRENGTH IS ENCOUNTERED; THEN THE STABILITY OF THE WINGWALLS IS SATISFACTORY.

**POROUS BACKFILL WITH FILTER FABRIC** 1'-6" THICK SHALL BE PLACED BEHIND THE WINGWALLS ONLY AND SHALL EXTEND TO 12" BELOW THE EMBANKMENT SURFACE. GEOTEXTILE FABRIC SHALL BE PLACED BETWEEN THE POROUS BACKFILL AND REPLACED EXCAVATION ADJACENT TO THE STRUCTURE. IT SHALL TURN UNDER THE BOTTOM OF THE POROUS BACKFILL AND RETURN 6" ABOVE THE TOP ELEVATION OF THE WEEPHOLE.

WEEPHOLES SHALL BE PLACED 6" TO 12" ABOVE THE NORMAL WATER ELEVATION OR GROUND LINE AND SHALL HAVE A MAXIMUM SPACING OF 10'-0". A MINIMUM OF ONE WEEPHOLE SHALL BE PROVIDED PER WINGWALL.

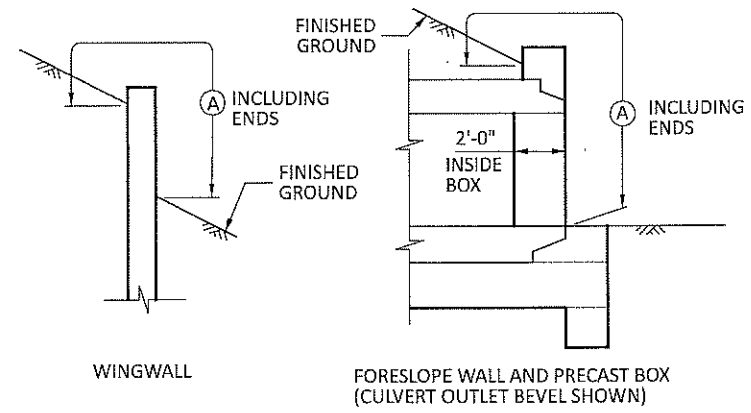
**PERFORMED EXPANSION JOINT FILLER:** PERFORMED EXPANSION JOINT FILLER (PEJF) CONFORMING TO CMS 705.03, 1 INCH THICK, SHALL BE PLACED ABOVE THE FOOTING BETWEEN THE SIDES OF THE BOX CULVERT AND THE ENDS OF THE WINGWALLS. PAYMENT FOR MATERIALS AND INSTALLATION SHALL BE INCLUDED WITH ITEM 516 - 1" PERFORMED EXPANSION JOINT FILLER.

**SEALING OF FORESLOPE WALL AND WINGWALLS:** ALL EXPOSED FORESLOPE WALL AND WINGWALL CONCRETE SHALL BE SEALED WITH EPOXY-URETHANE SEALER. THE LIMITS SHALL BE AS SHOWN IN THE DIAGRAMS BELOW. PAYMENT FOR THE EPOXY-URETHANE SEALER SHALL BE PER ITEM 512 - SEALING OF CONCRETE SURFACES.

**WATERPROOFING:** TYPE 2 WATERPROOFING, PER CMS 512.09 AND 711.25, SHALL EXTEND VERTICALLY DOWN THE ENTIRE SIDES OF THE PRECAST CULVERT SECTIONS FOR ALL PORTIONS OF THE CULVERT WHICH SHALL BE IN CONTACT WITH THE BACKFILL. PAYMENT FOR THE MEMBRANE WATERPROOFING SHALL BE AT THE CONTRACT PRICE BID PER SQUARE YARD FOR ITEM 512 - TYPE 2 WATERPROOFING.

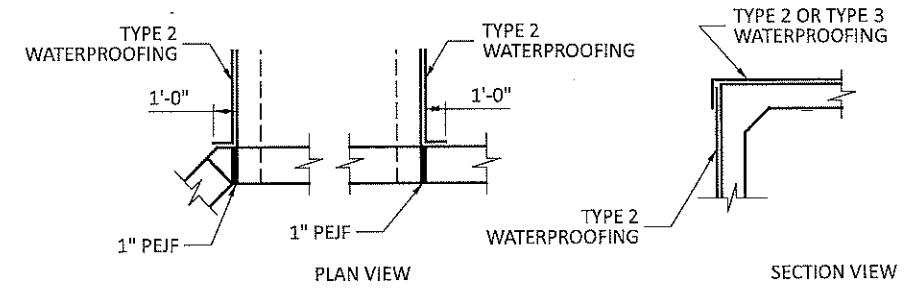
IF PAVEMENT IS NOT PLACED DIRECTLY ON TOP OF THE CULVERT, TYPE 2 WATERPROOFING, PER CMS 512.09 AND 711.25 SHALL BE APPLIED TO THE ENTIRE TOP SURFACE OF THE PRECAST CULVERT SECTIONS AND SHALL EXTEND ONE FOOT VERTICALLY DOWN THE SIDES FOR ALL PORTIONS OF THE CULVERT WHICH SHALL BE IN CONTACT WITH THE BACKFILL. PAYMENT FOR THE MEMBRANE WATERPROOFING SHALL BE AT THE CONTRACT PRICE BID PER SQUARE YARD FOR ITEM 512 - TYPE 2 WATERPROOFING.

IF PAVEMENT IS TO BE USED DIRECTLY ON TOP OF THE CULVERT, TYPE 3 WATERPROOFING, PER CMS 512.10 AND 711.29 SHALL BE APPLIED TO THE ENTIRE TOP SURFACE OF THE PRECAST CULVERT SECTIONS AND SHALL EXTEND ONE FOOT VERTICALLY DOWN THE SIDES FOR ALL PORTIONS OF THE CULVERT WHICH SHALL BE IN CONTACT WITH THE BACKFILL. PAYMENT FOR THE MEMBRANE WATERPROOFING SHALL BE AT THE CONTRACT PRICE BID PER SQUARE YARD FOR ITEM 512 - TYPE 3 WATERPROOFING.



**LIMITS OF ITEM 512-SEALING CONCRETE SURFACES**

(A) - SEAL ENTIRE CONCRETE SURFACE AREA



**WATERPROOFING DETAILS**

**BASIS OF PAYMENT:** ALL LABOR, EQUIPMENT AND INCIDENTALS REQUIRED TO CONSTRUCT THE FOOTING, CUTOFF WALL, WINGWALLS AND FORESLOPE WALL SHALL BE INCLUDED WITH ITEM 511 - CLASS C CONCRETE (RET WALL, WINGWALL INCLUDING FOOTING). PAYMENT FOR REINFORCING STEEL SHALL BE INCLUDED WITH ITEM 509 - EPOXY COATED REINFORCING STEEL.

*APPLICABLE SII ITEMS LISTED IN THE ESTIMATED QUANTITIES TABLE*

S10 10000 182 EACH DOWEL HOLES WITH NON-SHANK, NON-METALLIC GROUT

S11 47010 17 CY CLASS QC1 CONCRETE, CULVERT

ESTIMATED QUANTITIES				
ITEM	ITEM EXT	TOTAL	UNIT	DESCRIPTION
202	<del>11900</del>	11200	LUMP	STRUCTURE REMOVED - PORTIONS OF STRUCTURE REMOVED
503	11100		LUMP	COFFERDAMS, CRIBS, AND SHEETING AND EXCAVATION BRACING
503	<del>21400</del>	21300	LUMP	UNCLASSIFIED EXCAVATION (WINGWALL FOOTING)
509	10800	<del>10,567</del>	LB.	EPOXY COATED REINFORCING STEEL
511	46000	<del>61</del>	SQ. YD.	CLASS QC1 CONCRETE, RETAINING WALL OR WINGWALL, NOT INCLUDING FOOTING
511	46510	<del>49</del>	CU. YD.	CLASS QC1 CONCRETE, FOOTING
511	46610	<del>0.5</del>	CU. YD.	CLASS QC1 CONCRETE, HEADWALLS
512	33000	<del>50</del>	SQ. YD.	TYPE 2 MEMBRANE WATERPROOFING
516	13600	<del>38</del>	SQ. FT.	1" PERFORMED EXPANSION JOINT FILLER
518	21230		LUMP	POROUS BACKFILL WITH FILTER FABRIC - GEOTEXTILE
<del>601</del>	<del>11001</del>		SQ. YD.	RIPRAP USING 6" REINFORCED CONCRETE SLAB, AS PER PLAN
<del>611</del>	<del>96311</del>		LN. FT.	AS PER PLAN - XX'-0" SPAN X X'-0" RISE CONDUIT, TYPE A, 700-05, AS PER PLAN
<del>620</del>	<del>41200</del>		CU. YD.	LOW-STRENGTH MORTAR BACKFILL
512	10100	<del>46</del>	SQ. YD.	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)

**B** - TO BE USED FOR EXTENDING EXISTING BOX CULVERT

NOTE: TOTALS CARRIED TO GENERAL SUMMARY SHEET

DESIGN AGENCY

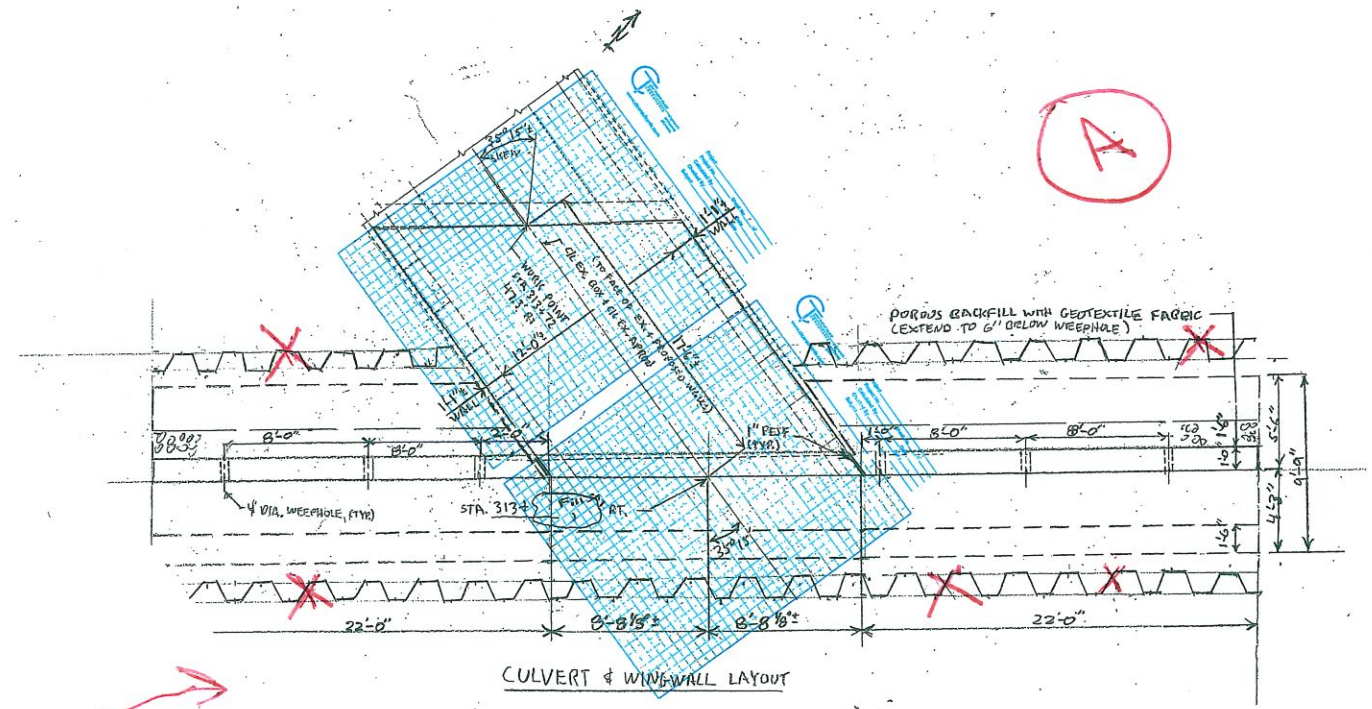
DESIGNER

REVIEWER

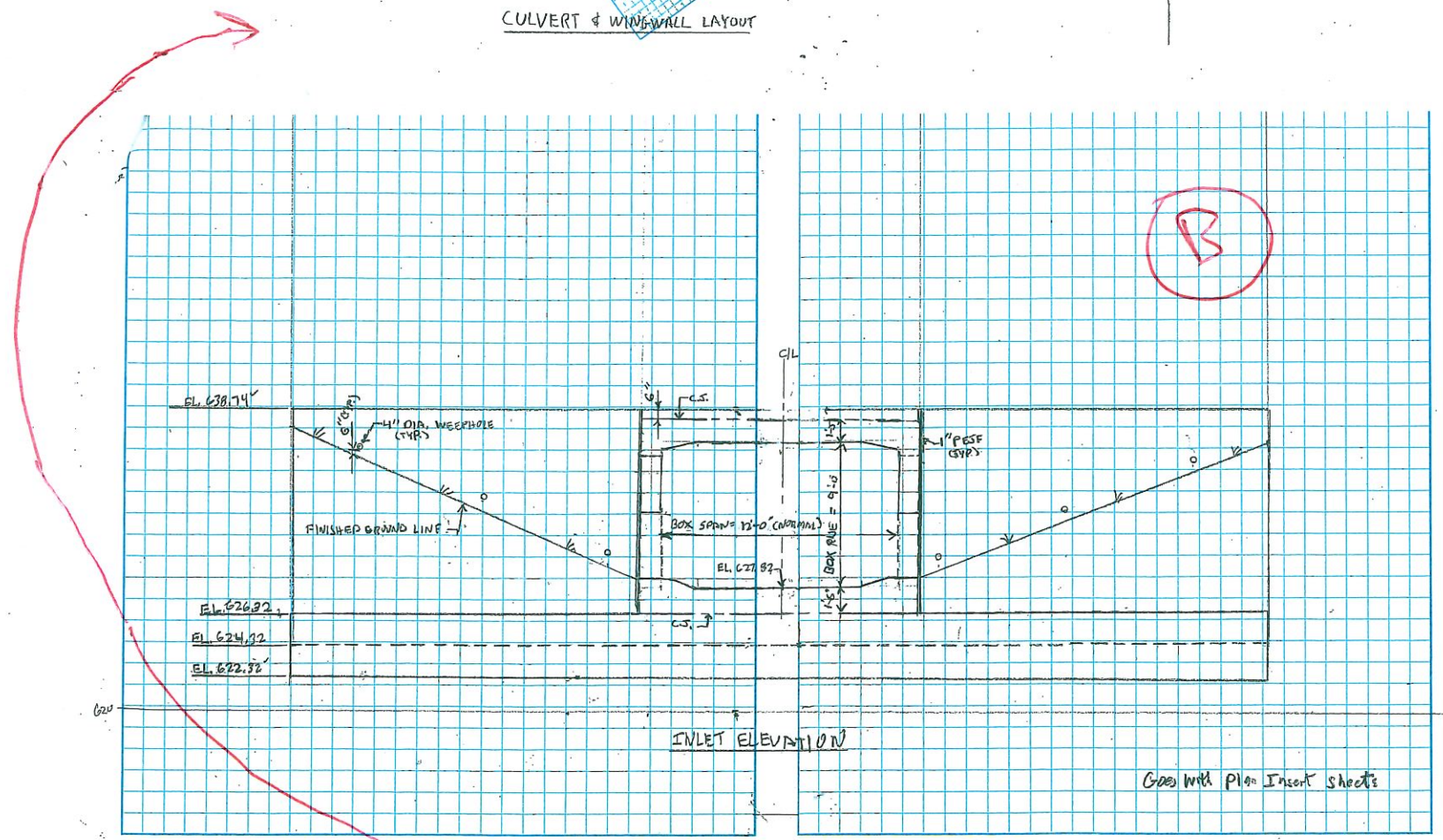
PROJECT ID

SUBSET	TOTAL
SHEET	TOTAL





INLET ELEVATION



CULVERT & WINGWALL LAYOUT

~~OUTLET ELEVATION~~

DESIGN AGENCY



DESIGNER

REVIEWER

PROJECT ID

SUBSET	TOTAL
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SHEET	TOTAL
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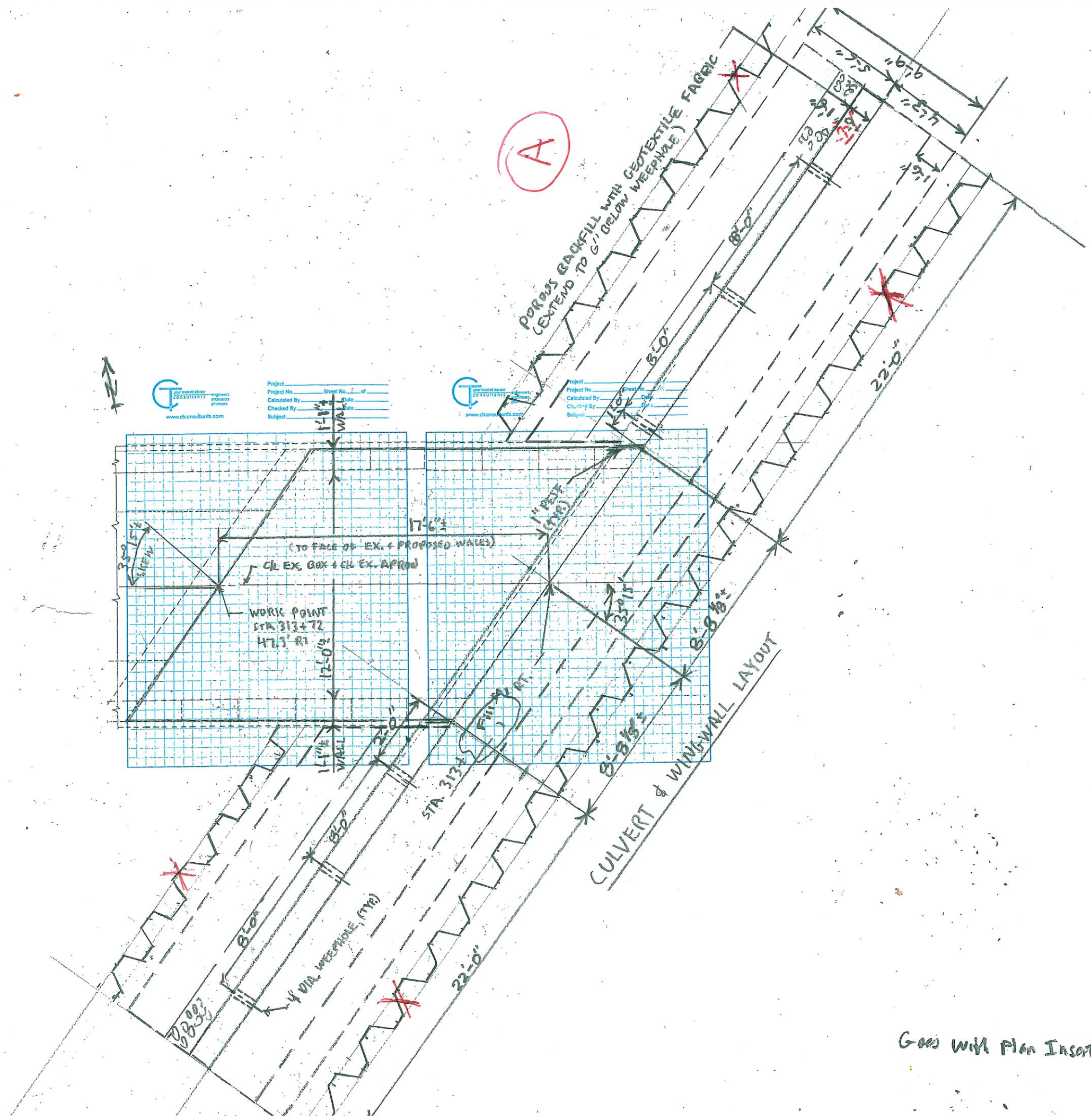
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Project No. \_\_\_\_\_ Sheet No. \_\_\_\_\_ of \_\_\_\_\_  
 Calculated By \_\_\_\_\_ Date \_\_\_\_\_  
 Checked By \_\_\_\_\_ Date \_\_\_\_\_  
 Subject \_\_\_\_\_

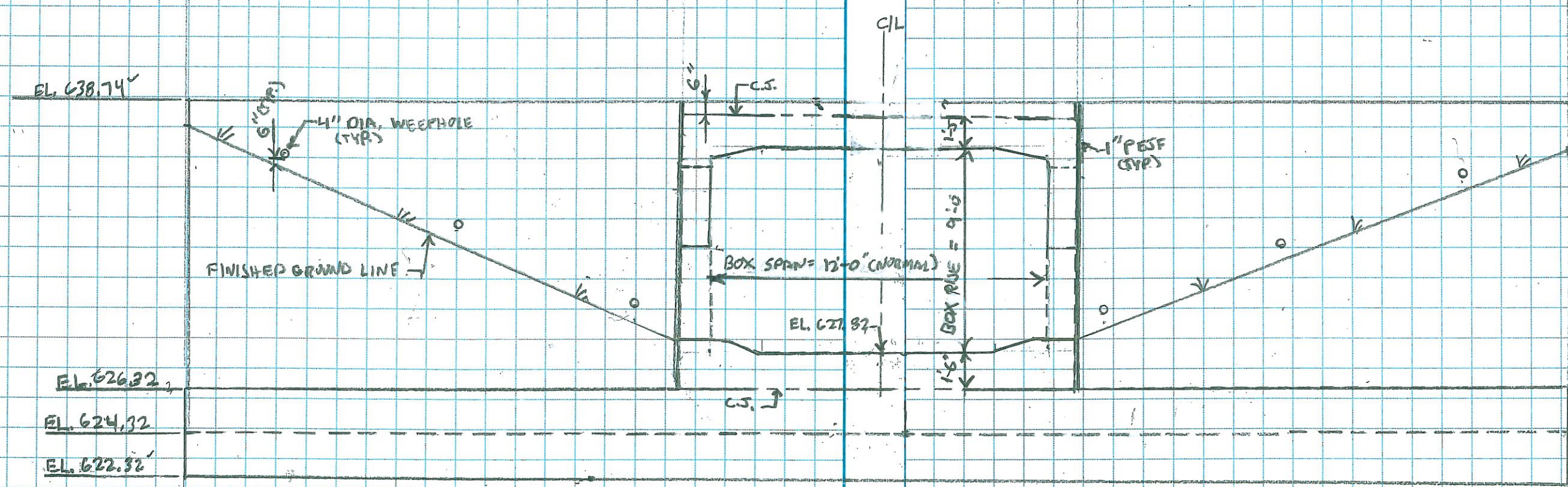


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 Checked By \_\_\_\_\_ Date \_\_\_\_\_  
 Subject \_\_\_\_\_



Go on with Plan Insert Sheets

(B)

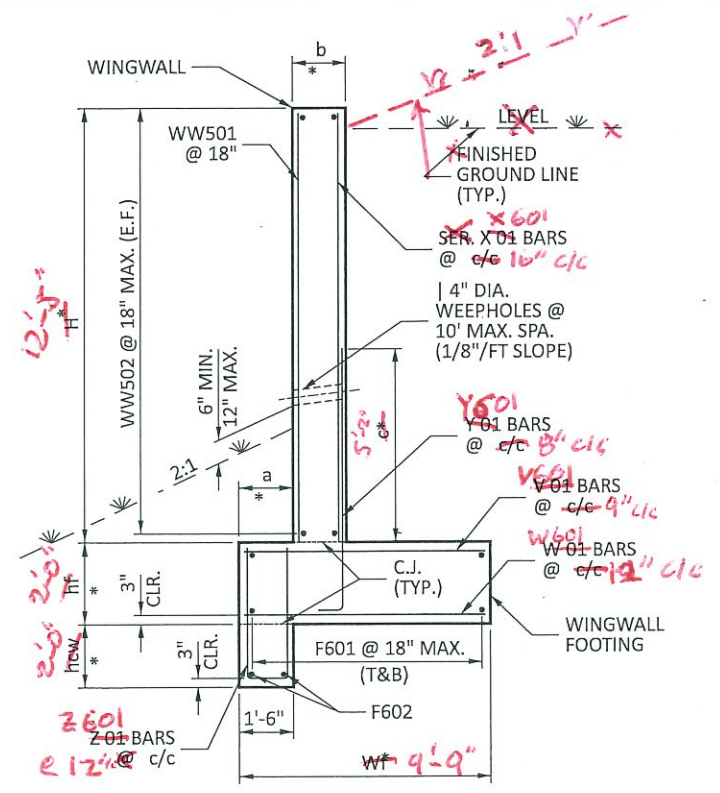
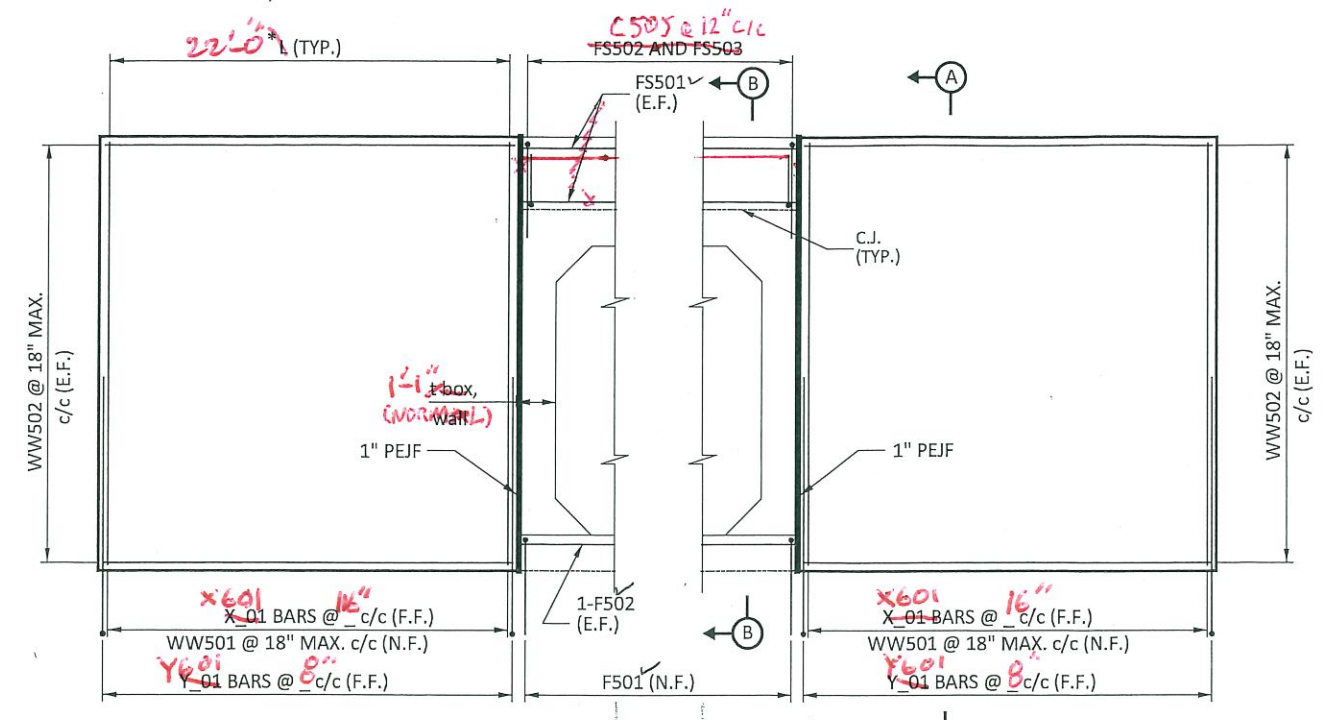


INLET ELEVATION

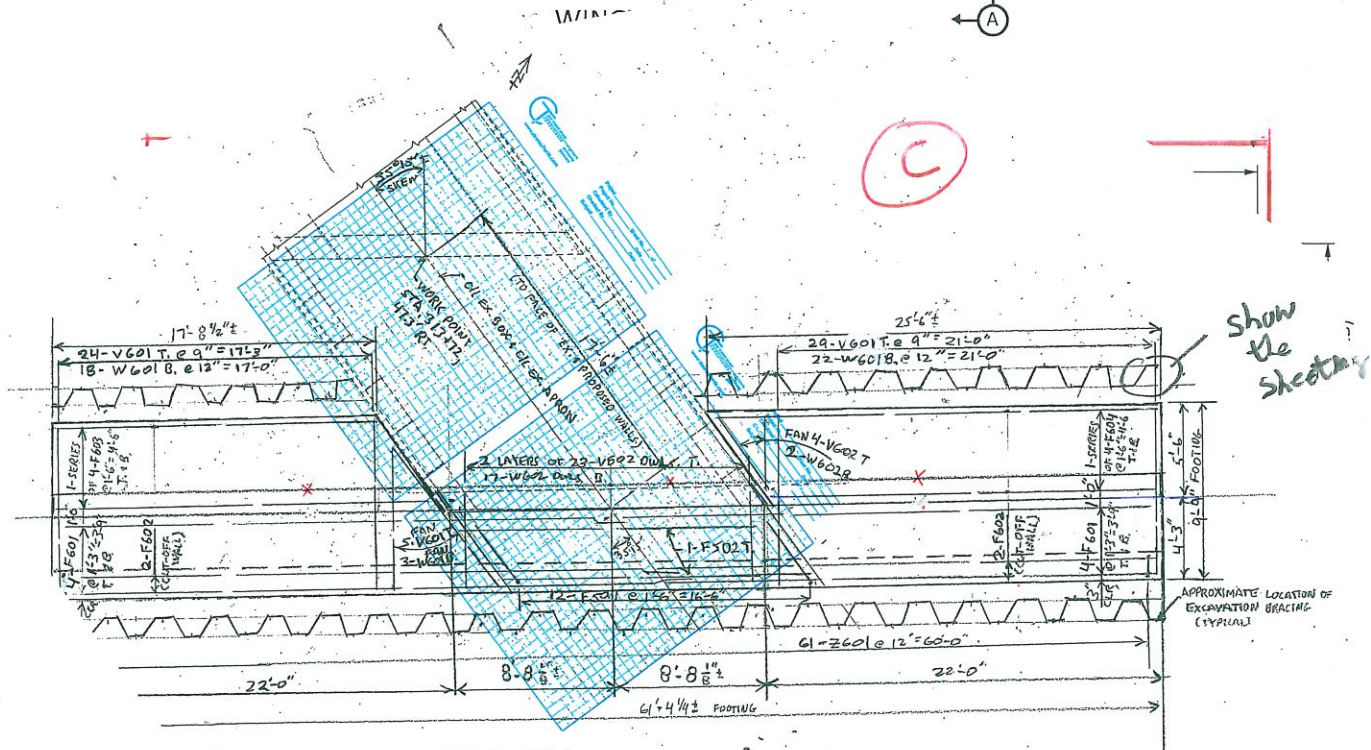
Goes with Plan Insert sheets



Goes with Plan Insert sheets



SECTION A-A  
(POROUS BACKFILL NOT SHOWN FOR CLARITY)



FOOTING PLAN  
(NOTE: THE PROPOSED BOX CULVERT RETROFIT ROOF SLAB IS NOT SHOWN FOR CLARITY.)

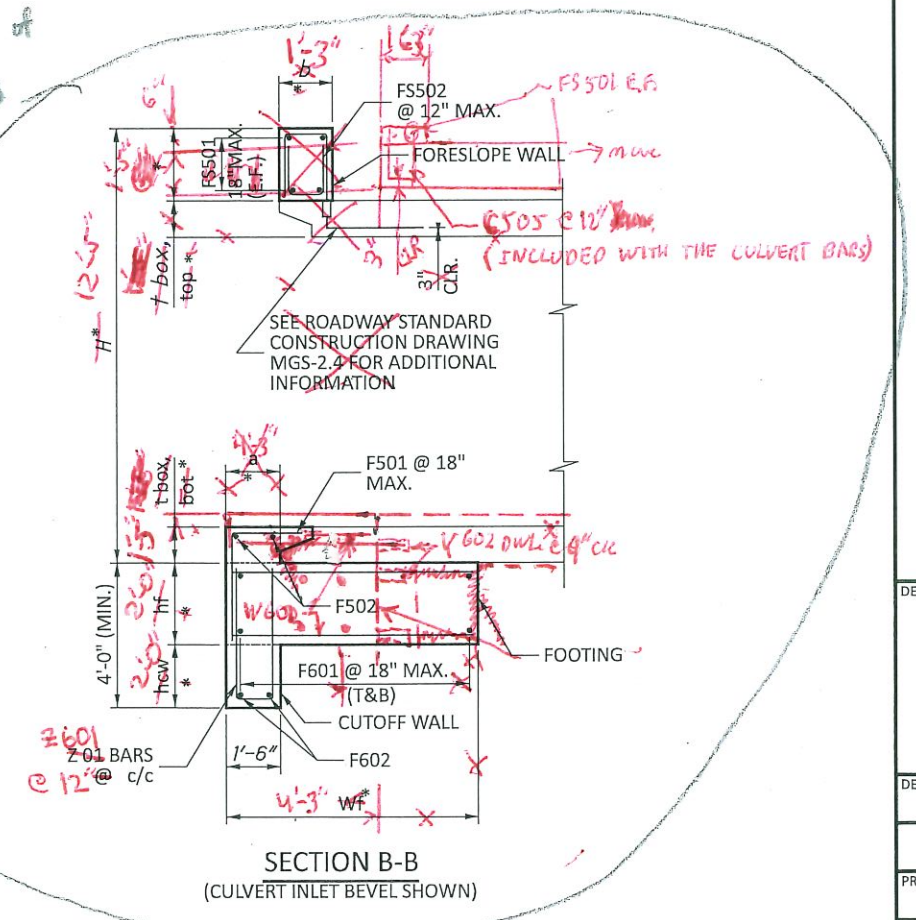
NOTES

- FOR CULVERT LOCATION PLAN, SEE SHEET ~~XX/XX-3261~~ **CAST-IN-PLACE BOX CULVERT RETROFIT DETAILS, SEE SHEET**
- FOR PRECAST BOX CULVERT DETAILS, SEE SHEET ~~XX/XX~~ **SEE SHEET**
- THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST DIGIT INDICATES THE BAR SIZE NUMBER. FOR EXAMPLE, WW501 IS A NO.5 BAR. BAR DIMENSIONS SHOWN ARE OUT TO OUT. ALL REINFORCING STEEL SHALL BE EPOXY COATED.
- THE LAP SPLICE LENGTHS USED IN THESE DETAILS ARE AS FOLLOWS: 2'-5" FOR #5 BARS; 2'-11" FOR #6 BARS.

C.J.	CONSTRUCTION JOINT	N.F.	NEAR FACE
CL.R.	CLEAR	SER.	SERIES
DIA.	DIAMETER	STR.	STRAIGHT
E.F.	EACH FACE	(T)	TOP
F.F.	FAR FACE	(B)	BOTTOM
MAX.	MAXIMUM	T&B	TOP AND BOTTOM
MIN.	MINIMUM	TYP.	TYPICAL
PEJF	PREFORMED EXPANSION JOINT FILLER	INC.	INCREMENT

\*AN UNDERSCORE ( ) OR AN ASTERISK ( ) INDICATES A SPACE TO BE COMPLETED BY DESIGNER USING INFORMATION PICKED FROM THE STANDARD DRAWINGS.

Mirror Image of Section B-B drawn for the Culvert Retrofit



SECTION B-B  
(CULVERT INLET BEVEL SHOWN)

$tan(55.23^\circ) = \frac{66}{x}$   
 $x = 46.64'$   
 $41.60' \cos(55.23^\circ) = 29.84'$   
 $29.84' + 16' = 45.84'$   
 $46.64' - 45.84' = 0.80'$   
 $tan(10.8^\circ) = \frac{0.80}{x}$   
 $x = 4.26'$   
 $46.64' + 4.26' = 50.90'$   
 $50.90' \cos(10.8^\circ) = 49.85'$   
 $49.85' - 46.64' = 3.21'$   
 $3.21' \tan(10.8^\circ) = 0.61'$   
 $0.61' + 46.64' = 47.25'$   
 $47.25' \cos(10.8^\circ) = 46.64'$   
 $46.64' \sin(10.8^\circ) = 8.85'$   
 $8.85' + 17.35' = 26.20'$   
 $26.20' \cos(10.8^\circ) = 25.64'$   
 $25.64' - 17.35' = 8.29'$   
 $8.29' \sin(10.8^\circ) = 1.56'$   
 $1.56' + 8.85' = 10.41'$   
 $10.41' \cos(10.8^\circ) = 10.20'$   
 $10.20' - 8.85' = 1.35'$

(1.57)  $tan(55.23^\circ) = \frac{19.08}{x}$   
 $x = 17.35'$   
 $tan(10.8^\circ) = \frac{8.85}{x}$   
 $x = 8.29'$   
 $17.35' + 8.29' = 25.64'$   
 $25.64' \cos(10.8^\circ) = 25.16'$   
 $25.16' - 17.35' = 7.81'$   
 $7.81' \sin(10.8^\circ) = 1.47'$   
 $1.47' + 8.85' = 10.32'$   
 $10.32' \cos(10.8^\circ) = 10.13'$   
 $10.13' - 8.85' = 1.28'$

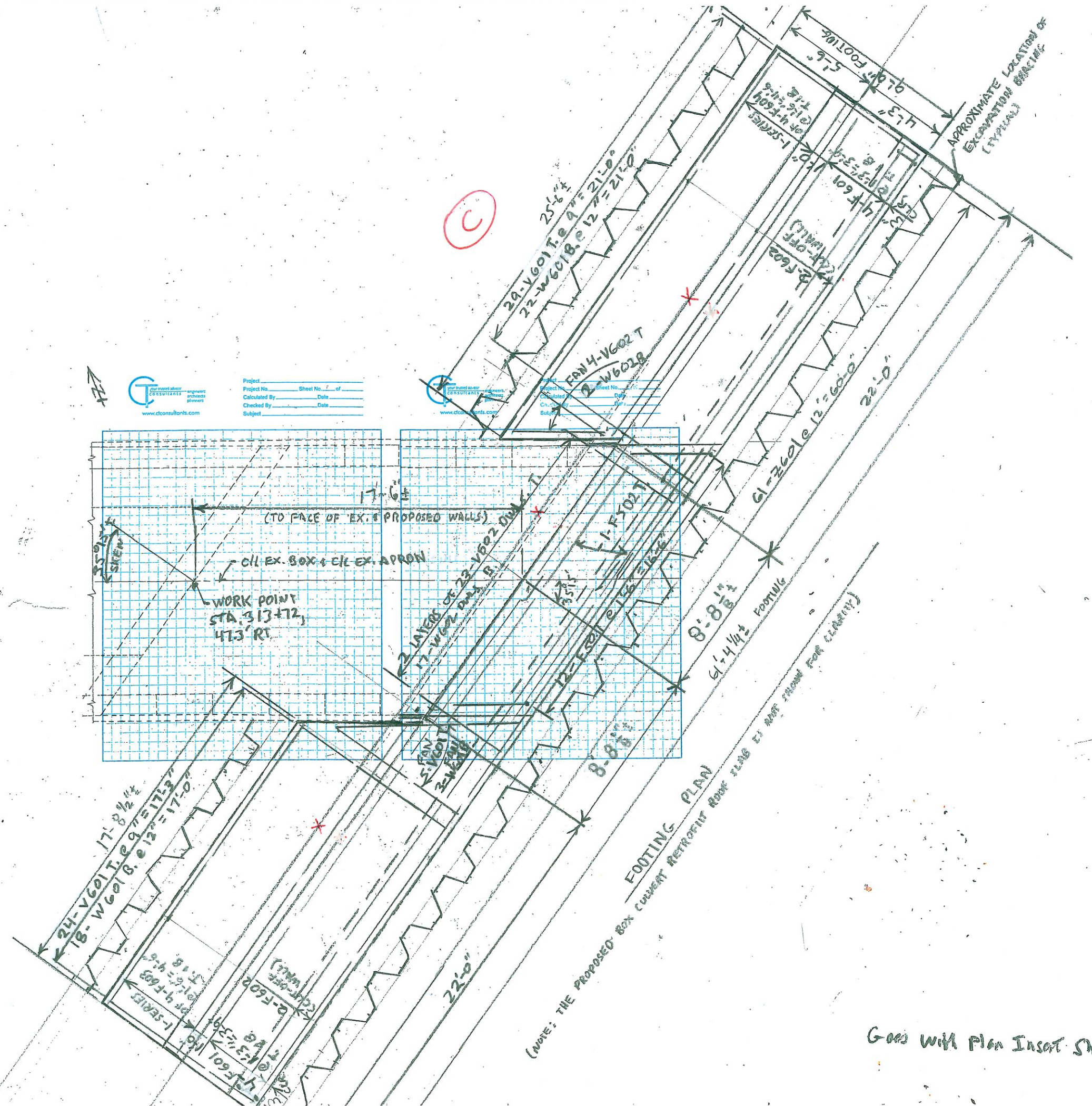
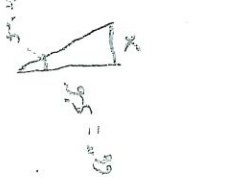
$22.0' \sin(8.8^\circ) = 3.44'$   
 $3.44' + 17.56' = 21.00'$   
 $21.00' \cos(8.8^\circ) = 20.61'$   
 $20.61' - 17.56' = 3.05'$   
 $3.05' \sin(8.8^\circ) = 0.47'$   
 $0.47' + 17.56' = 18.03'$

$17.56' \sin(8.8^\circ) = 2.70'$   
 $2.70' + 14.86' = 17.56'$   
 $17.56' \cos(8.8^\circ) = 17.23'$   
 $17.23' - 14.86' = 2.37'$   
 $2.37' \sin(8.8^\circ) = 0.37'$   
 $0.37' + 14.86' = 15.23'$

$17.56' \cos(8.8^\circ) = 17.23'$   
 $17.23' - 14.86' = 2.37'$   
 $2.37' \sin(8.8^\circ) = 0.37'$   
 $0.37' + 14.86' = 15.23'$   
 $15.23' \cos(8.8^\circ) = 15.00'$   
 $15.00' - 14.86' = 0.14'$   
 $0.14' \sin(8.8^\circ) = 0.02'$   
 $0.02' + 14.86' = 14.88'$

$tan(55.23^\circ) = \frac{66}{x}$   
 $x = 46.64'$   
 $46.64' \sin(55.23^\circ) = 38.78'$   
 $38.78' + 17.56' = 56.34'$   
 $56.34' \cos(55.23^\circ) = 32.10'$   
 $32.10' - 17.56' = 14.54'$   
 $14.54' \sin(55.23^\circ) = 11.98'$   
 $11.98' + 38.78' = 50.76'$   
 $50.76' \cos(55.23^\circ) = 28.84'$   
 $28.84' - 17.56' = 11.28'$

$tan(55.23^\circ) = \frac{66}{x}$   
 $x = 46.64'$   
 $46.64' \sin(55.23^\circ) = 38.78'$   
 $38.78' + 17.56' = 56.34'$   
 $56.34' \cos(55.23^\circ) = 32.10'$   
 $32.10' - 17.56' = 14.54'$   
 $14.54' \sin(55.23^\circ) = 11.98'$   
 $11.98' + 38.78' = 50.76'$   
 $50.76' \cos(55.23^\circ) = 28.84'$   
 $28.84' - 17.56' = 11.28'$



CONSULTANTS  
 Project No. \_\_\_\_\_ Sheet No. \_\_\_\_ of \_\_\_\_  
 Calculated By \_\_\_\_\_ Date \_\_\_\_\_  
 Checked By \_\_\_\_\_ Date \_\_\_\_\_  
 Subject \_\_\_\_\_



Goes with Plan Insert Sheets

TYPE C HEADWALL REINFORCING SCHEDULE

BAR		BAR TYPE DIMENSIONS									
MARK	NUMBER	LENGTH	WEIGHT	TYPE	A	B	C	D	E	R	INCL
WINGWALLS											2,591
XG01	34	12'-3"	626-	STR.							
YG01	86	7'-9"	768-	I	1'-0"	6'-11"					
WW501	30	12'-3"	383-	STR.							
WW502	36	21'-8"	814-	STR.							
FOOTINGS & CUTOFF WALL											4,358
VG01	58	9'-3"	806-824	STR.							
VG02	50	5'-0"	376-	STR.							
WG01	43	9'-5"	597-609	STR.							
WG02	19	5'-0"	143-	STR.							
ZG01	561	8'-2"	534	S	3'-7"	1'-0"	3'-7"				
FG01	12	9'-9"	122-	I	4'-0"	5'-0"					
FS02	2	17'-0"	35-	STR.							
FG01	16	31'-11"	768-	STR.							
FG02	4	31'-11"	192	STR.							
FG03	2-SER.	17'-2"	224-	STR.							1'-0"
	OF	TO									
	4	20'-2"									
FG04	2-SER.	25'-0"	318-	STR.							1'-0"
	OF	TO									
	4	28'-0"									
FORESLOPE WALL											36
FS01	2	17'-0"	35-	STR.							
TOTAL = <del>6,259</del> LBS											6,985



Project LNK-20-1408  
 Project No. 180700 Sheet No.      of       
 Calculated By DFR Date 2/11/22  
 Checked By      Date       
 Subject Headwall, Culvert @ STA 313+72, RT

INSERT ODOT BOX  
CULVERT REINFORCING  
DESIGN HERE IF SPAN > 12'

*Not Needed (12' span ≠ 12' span)*

← This option gives a limited check

Insert the Bar Schedule

Generated from Design Data Sheet

HWDD-Rebar 01-17-20 MS EXCEL

Sheet (for Bar Schedules) → Input Values on Next page →

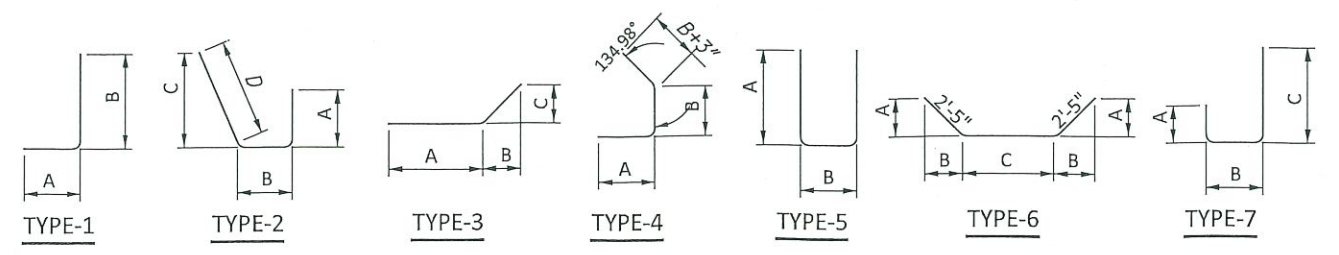
**TYPE C HEADWALL** then edit

← for my Bar Schedule

OR

← Copy & Paste the Bar Schedule from STA 149+38 Culvert and revise to my schedule

← This option does not check



DESIGN AGENCY	
DESIGNER	
REVIEWER	
PROJECT ID	
SUBSET	TOTAL
SHEET	TOTAL

2/7/22, 3:25 PM

**Type C Headwall: Reinforcing Schedule Calculations**

If Override Option is Not Used, Leave Cell Blank \*

Reference shall be made to the "Concrete Headwalls for Precast Box Culverts" Standard Drawing for headwall dimensions and reinforcing data.

**Precast Box Culvert:**

Span = ~~10.00 ft~~ **12.00 ft**  
 Rise = ~~7.00 ft~~ **9.00 ft**  
 Default      Override \*  
 $t_{box,wall}$  = 10.00 in ~~13.00 in~~  
 $t_{box,top}$  = 10.00 in ~~18.00 in~~  
 $t_{box,bot}$  = 10.00 in ~~17.00 in~~

**Wingwalls:**

H = ~~10.50 ft~~ **12.42 ft**  
 $h_1$  = ~~10.50 ft~~  
 $h_2$  = ~~10.50 ft~~ ( $h_2=h_1=H$ )  
 $L_1$  = ~~18.00 ft~~ **22.00 ft**  
 $L_2$  = ~~18.00 ft~~ ( $L_2=L_1$ )  
 $a$  = ~~3.07 ft~~ **4.25 ft**  
 $b$  = ~~1.25 ft~~  
 $c$  = ~~3.92 ft~~ **5.17 ft**

**Footing & Cutoff Wall:**

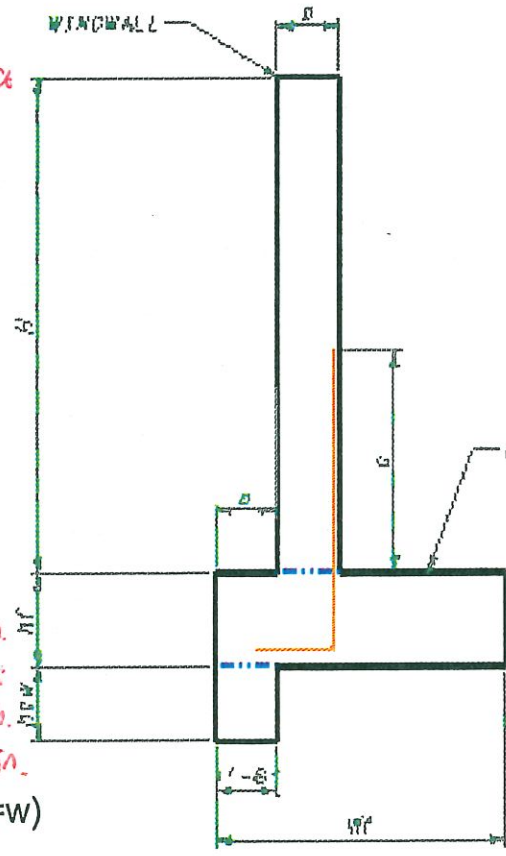
$W_f$  = ~~8.00 ft~~ **9.75 ft**  
 $h_f$  = ~~2.00 ft~~  
 $h_{cw}$  = ~~2.00 ft~~

**Bar Size:**

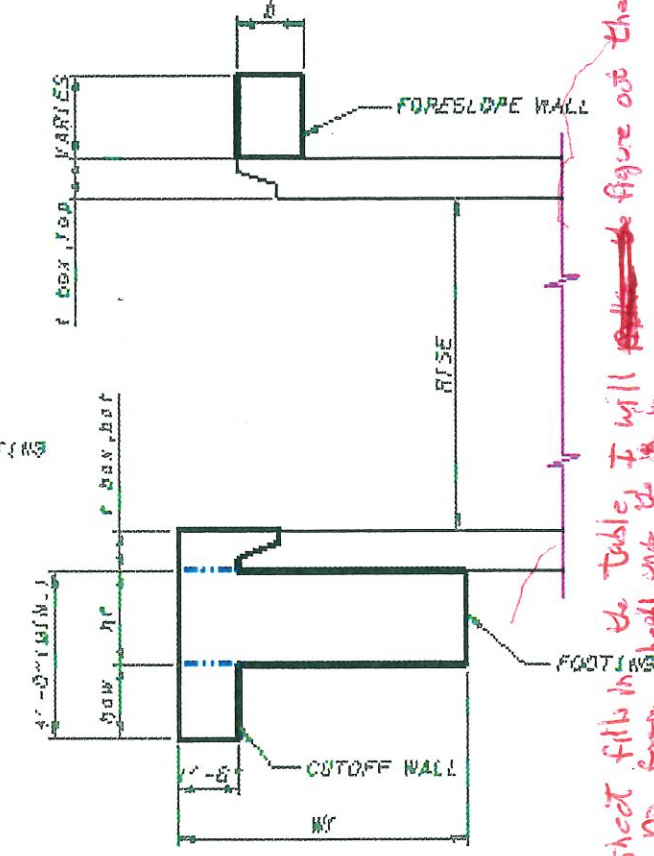
V = #5-#6  
 W = #5-#6  
 X = #5-#6  
 Y = #6  
 Z = #5

**Bar Spacing:**

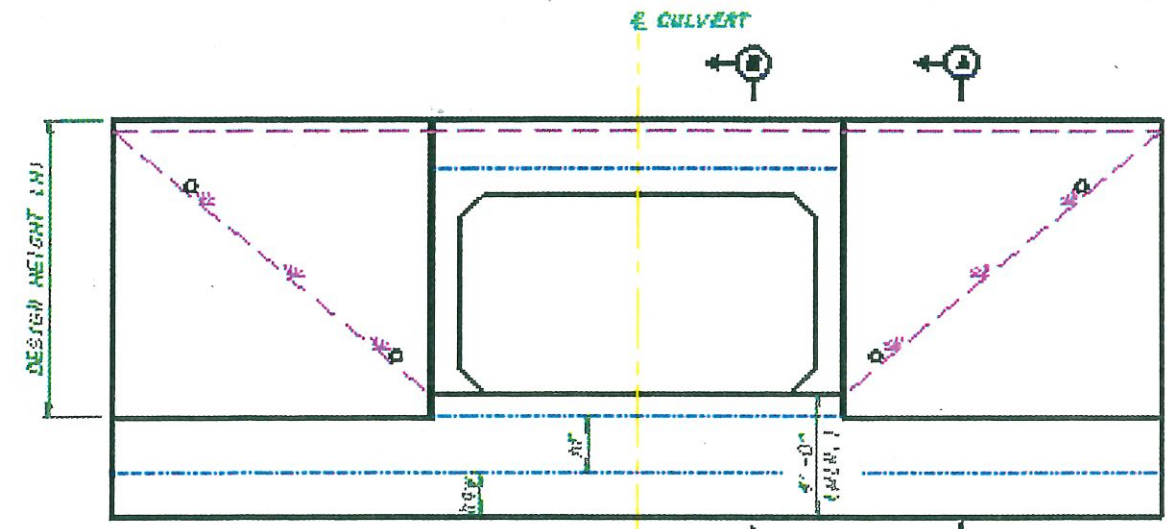
v = ~~18.00 in~~ **6.00 in**  
 w = ~~18.00 in~~ **9.00 in**  
 x = ~~18.00 in~~ **16.00 in**  
 y = ~~9.00 in~~ **8.00 in**  
 z = 18.00 in ( $z=w$ )



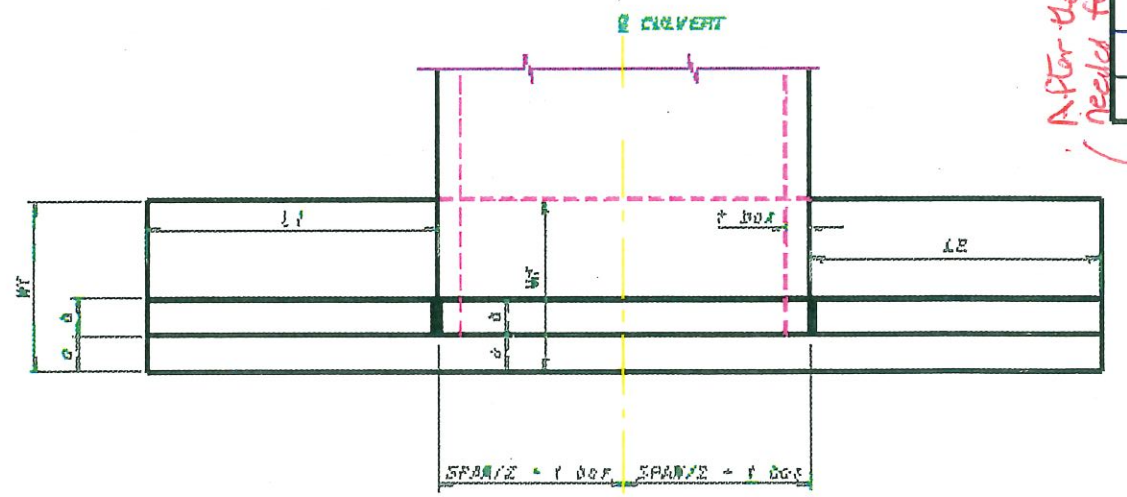
SECTION A-A



SECTION B-B



ELEVATION TYPE C HEADWALL



PLAN TYPE C HEADWALL

After the spreadsheet fills in the table, I will figure out the remaining needed for having no footing under the box.

BAR MARK
X501
Y601
WW501
WW502
V501
W501
Z501
F501
F502
F601
F602
FS501
FS502
FS503

If Override Option is Not Used, Leave Cell Blank \*

Culverts" Standard Drawing for headwall dimensions and reinforcing data.

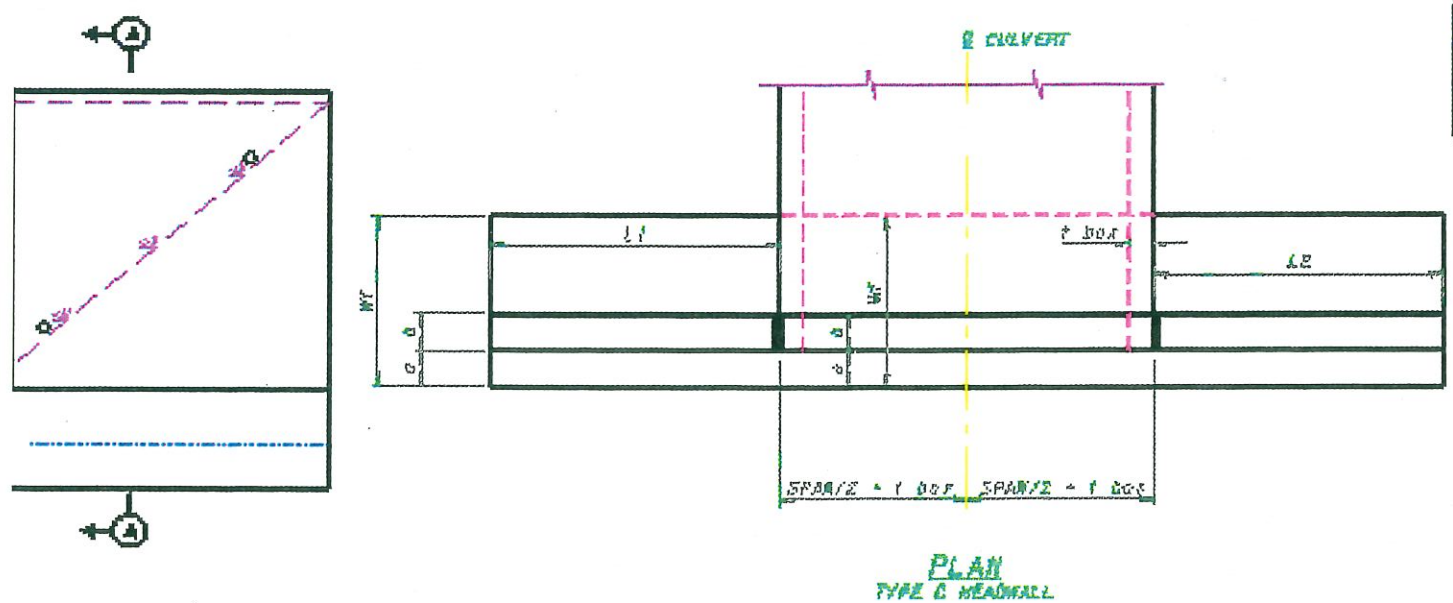
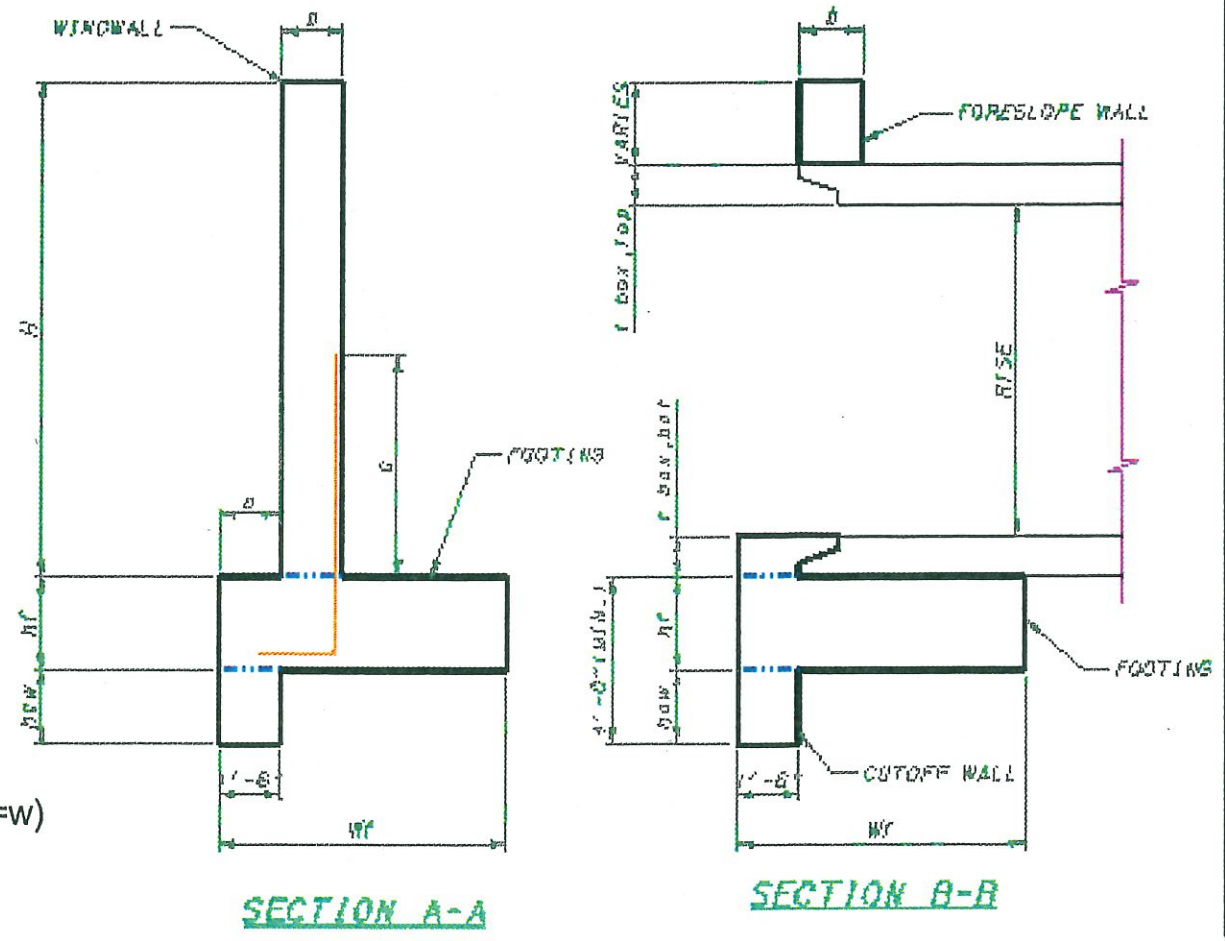
**Cutoff Wall:**

- 8.00 ft
- 2.00 ft
- 2.00 ft

- #5
- #5
- #5
- #6
- #5

**ig:**

- 18.00 in
- 18.00 in
- 18.00 in
- 9.00 in
- 18.00 in (z=w)



TYPE C HEADWALL REINFORCING SCHEDULE								
BAR MARK	NUMBER	LENGTH	WEIGHT (LBS.)	TYPE	BAR TYPE DIMENSIONS			
					A	B	C	
<b>WINGWALLS</b>								
X501	26	10'- 4"	281	STR.				
Y601	50	6'- 6"	489	1	1'- 0"	5'- 8"		
WW501	26	10'- 4"	281	STR.				
WW502	32	17'- 8"	590	STR.				
<b>FOOTING &amp; CUTOFF WALL</b>								
V501	33	7'- 8"	264	STR.				
W501	33	7'- 8"	264	STR.				
Z501	33	8'- 2"	282	5	3'- 7"	1'- 2"		
F501	9	6'- 1"	58	1	3'- 8"	2'- 6"		
F502	2	11'- 4"	24	STR.				
F601	28	24'- 11"	1,048	STR.				
F602	4	24'- 11"	150	STR.				
<b>FORESLOPE WALL</b>								
FS501	4	11'- 4"	48	STR.				
FS502	13	3'- 8"	50	5	1'- 6"	0'- 11"		
FS503	13	4'- 5"	60	7	1'- 6"	0'- 11"	2'- 3"	
<b>TOTAL</b>			<b>3,889</b>					