

MAHONING COUNTY
MAH-18-15.50

Soundings Legend
● = Core Boring

FOUNDATION SOUNDINGS: Foundation design & foundation quantities are based on a study of borings and soil sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

BENCH MARK #216 Elev. 990.634
N. Bolt Hydrant, N.W. Corner Oregon and Schenley
78' W on Oregon from Schenley

BENCH MARK #218 Elev. 1001.203
W. Bolt Hydrant, One Block N. of Oregon

PROPOSED STRUCTURES

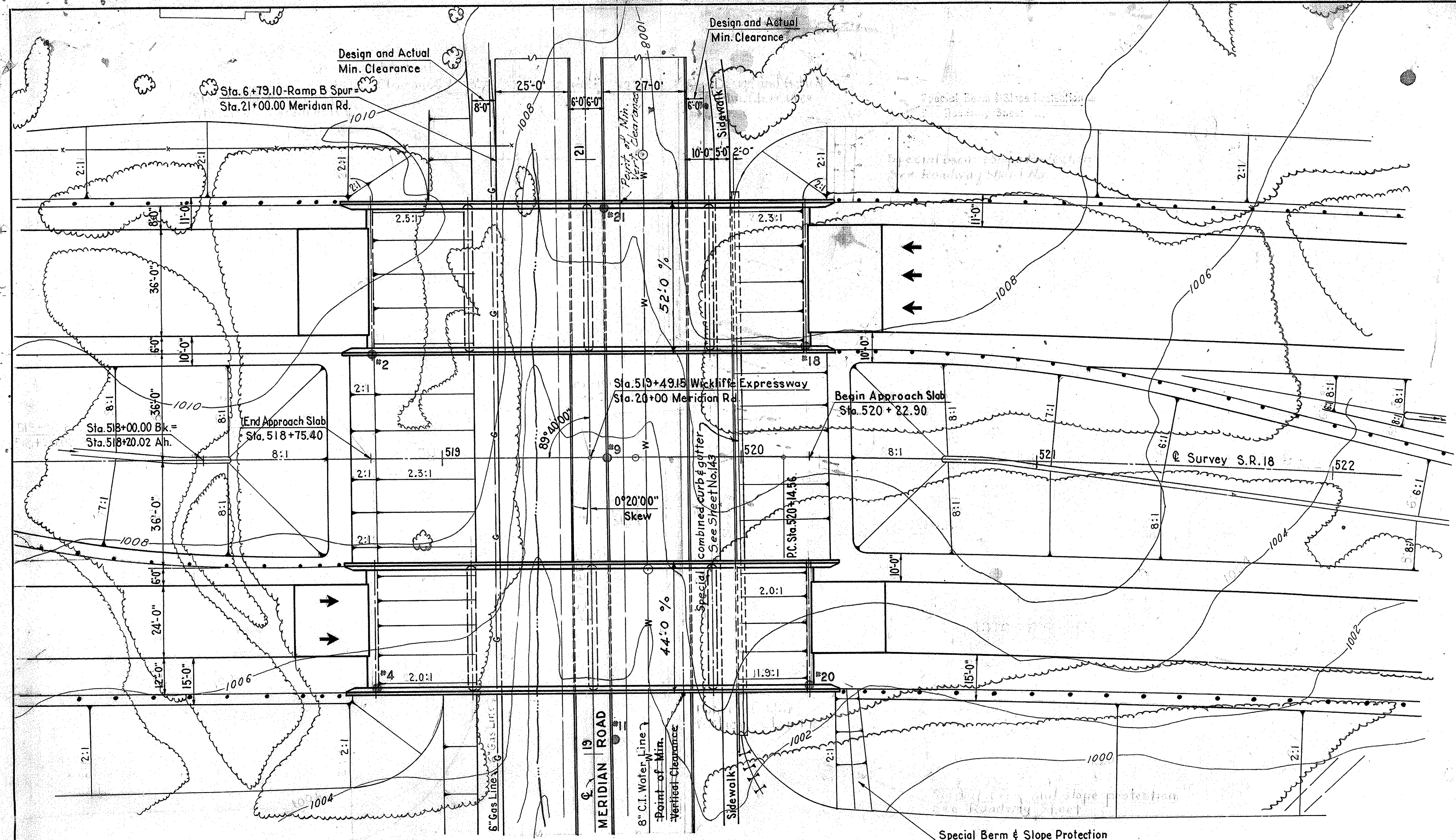
TYPE: Continuous reinforced slab with reinforced concrete substructure
SPANS: 32'-6", 40'-6", 40'-6", 32'-0"
ROADWAY: Left 50' f/f, Right 42' f/f parapets with 1'-2" curbs
LOAD FREQUENCY: CF 2000 (57) Adequate for alternate AASHTO Loading
SKEW: 0° 20' Rt. forward
WEARING SURFACE: 1" Monolithic Concrete
APPROACH SLABS: AS-1-54 (25' Long)
ALIGNMENT: Tangent
SUPERELEVATION: Varies

1975 A.D.T. = 45,935

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SITE PLAN
BRIDGE NO. MAH-18-1552 L. & R
OVER MERIDIAN ROAD
MAHONING COUNTY
STA. 518+75.40 TO STA. 520+22.90

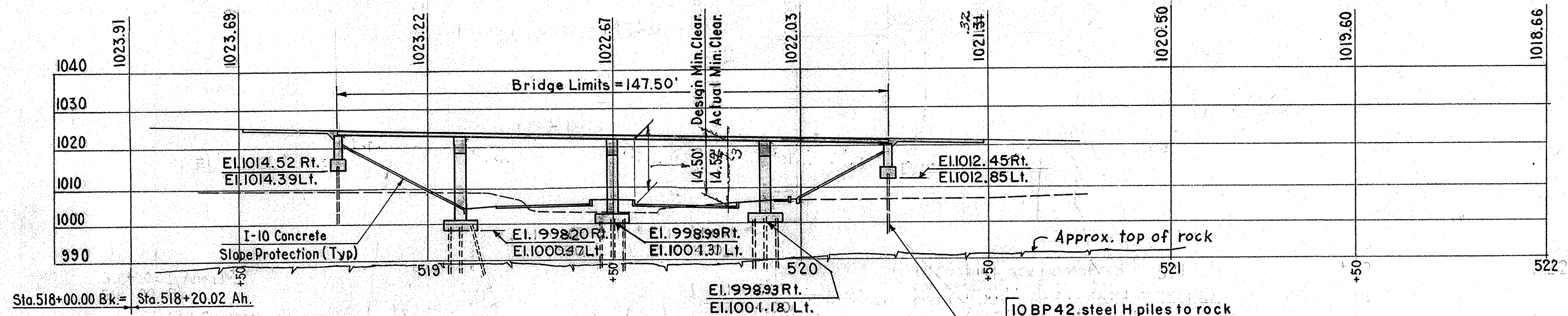
PRESENT TOPOGRAPHY		PROPOSED WORK	
SURVEYED	DRAWN	DESIGNED	DRAWN
Aurilio	T.G.	L.G.H.	Bo
		REVIEWED	REVIEWED
		R.L.M.	R.L.M.



PLAN

Special Berm & Slope Protection
See Roadway Sheet No. 142

CURVE DATA
P.I. Sta. 527 + 73.91
Δ - 22° 00' Rt.
D - 1° 28'
R - 3906.53'
T - 759.35'
L - 1500.00'
E - 73.12'
P.C. Sta. 520 + 14.56 (84'-0" Median)
P.T. Sta. 535 + 14.56 (40'-0" Median)
S.E. 024' / ft.



PROFILE ALONG C S.R. 18

10 BP42 steel H piles to rock
Estimated average pay length
Abutment 1 Rt. & 1 Lt. 27'-0"
Pier 1 Lt. 17'-0"; Pier 1 Rt. 14'-0"
Pier 2 Lt. 19'-0"; Pier 2 Rt. 17'-0"
Pier 3 Lt. 17'-0"; Pier 3 Rt. 17'-0"
Abut. 2 Lt. 21'-0"; Abut. 2 Rt. 25'-0"

Rev. 2-1-66

MAHONING CO.
MAH-18-15.50

GENERAL NOTES

MERIDIAN ROAD

REFERENCE shall be made to Standard Drawing AS-1-54 revised July 5, 1962 and to Supplemental Specifications S-101 "Water-reducing, Set-retarding Admixture," dated July 12, 1962.

DESIGN SPECIFICATION: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated September 1, 1957, together with current revisions thereof.

EXCAVATION QUANTITY includes the removal of fill material required for construction of abutments. EXCAVATION QUANTITY includes the removal of fill material required for construction of Pier 2, left and right.

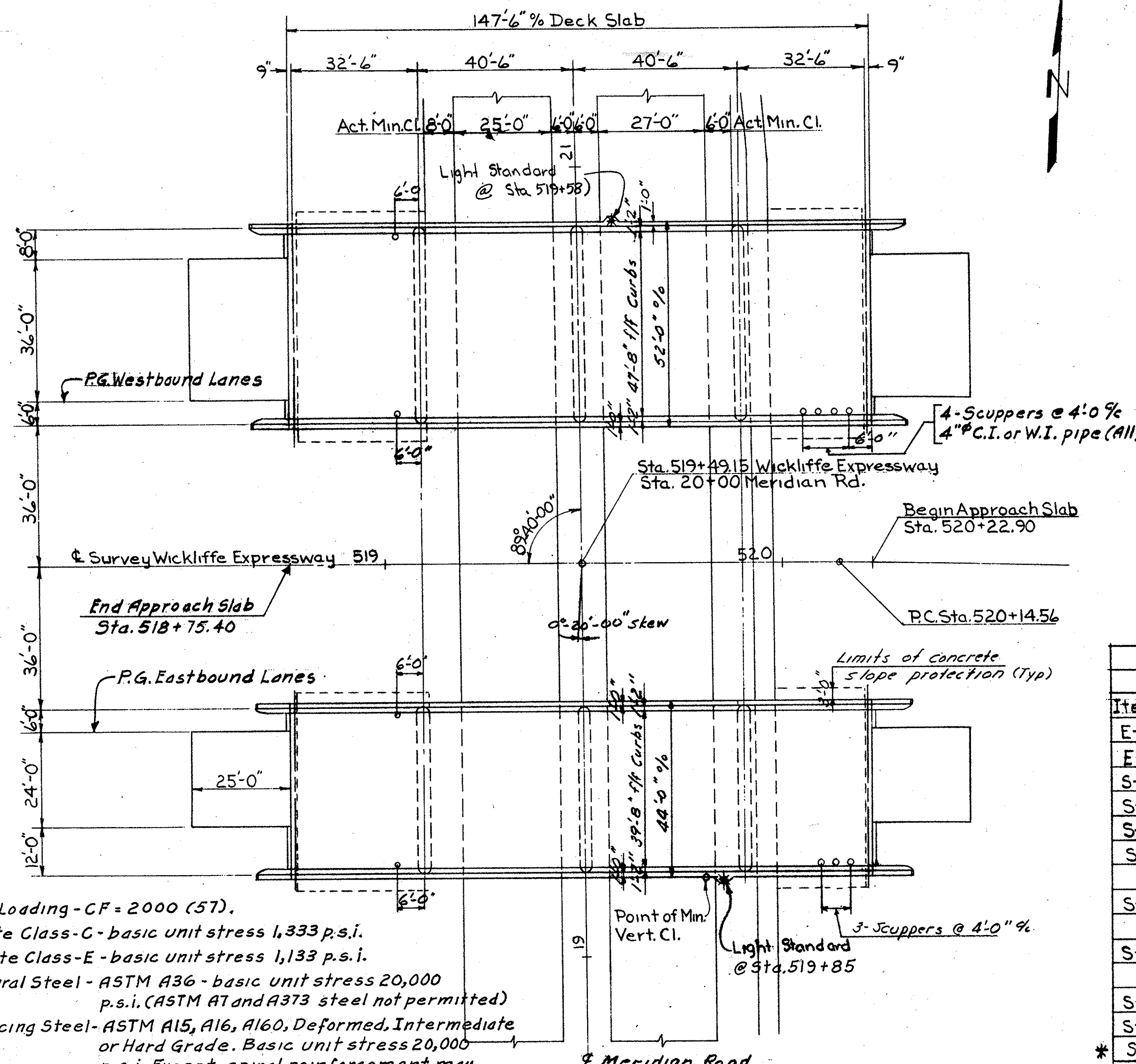
PILES shall be driven with a hammer of not less than 11,000 ft. lb. per blow to firm contact with rock. If the length of penetration is approximately equal to the depth to rock according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Sec. S-18.05 is not less than the following value for a pile hammer of the indicated energy rating:

	11,000 ft. lb.	15,000 lb. ft.
Abutment 1R, 1L and 2R	47T	40T
Pier 1L, 2R, 3L and 3R	43T	39T
Pier 1R	47T	42T
Pier 2L	41T	37T
Abutment 2L	50T	43T

if the energy rating of the hammer is between the ratings as shown above, the required formula capacity shall be determined by interpolation. The design load is 35 tons per pile for abutment piles and 31 tons per pile for the pier piles.

CAMBER: Loaded falsework shall have a camber of 1/2" for end spans and 1/16" for the middle spans to allow for vertical curve and dead load deflection. This is the amount of camber required before falsework is released. To obtain this proper allowance shall be made for the deflection of falsework members.

MACHINE FINISH: At the Contractor's option, the concrete bridge deck may be finished by the use of a finishing machine.

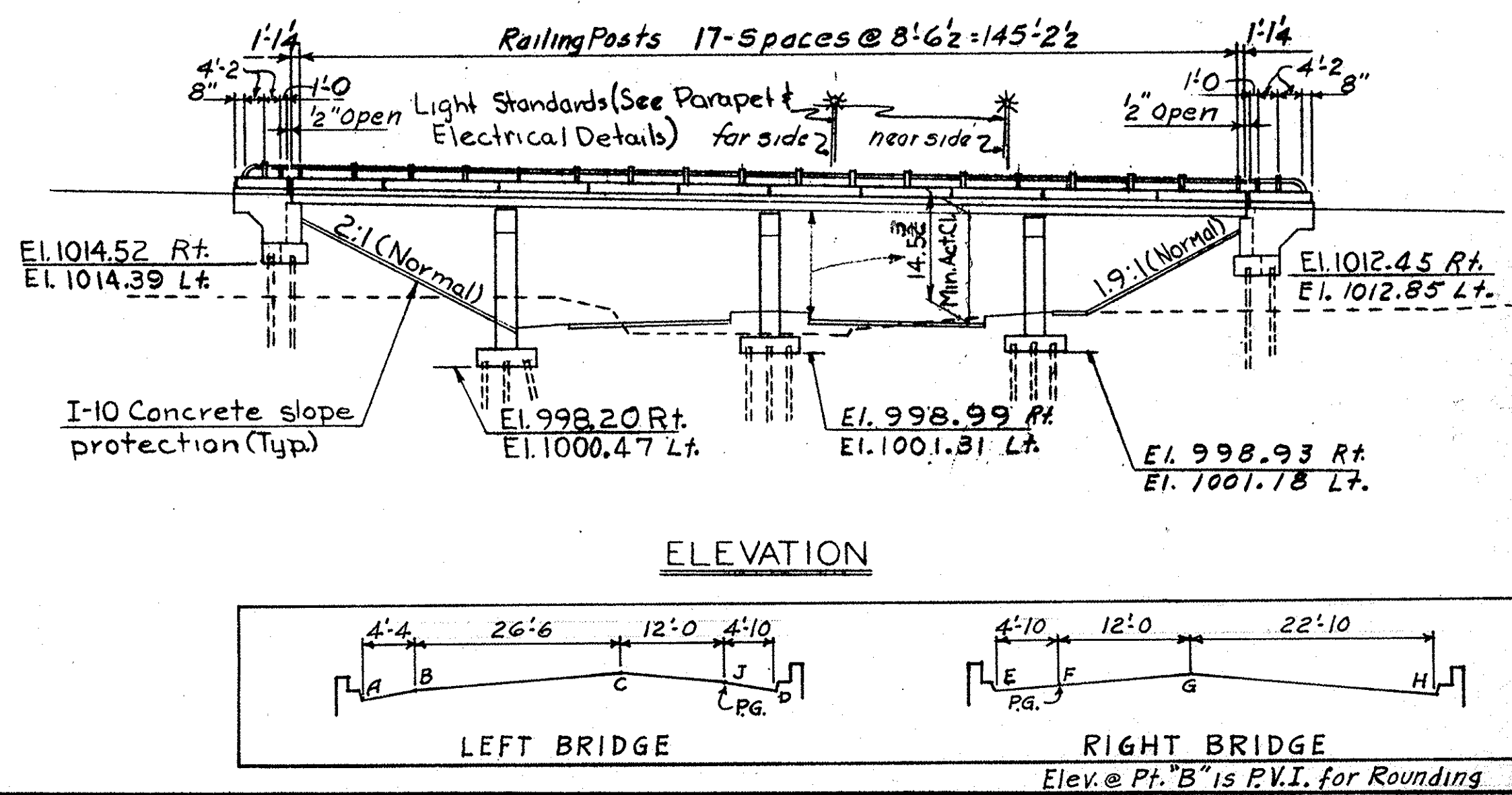


Design Loading - CF = 2000 (S7).
Concrete Class-C - basic unit stress 1,333 p.s.i.
Concrete Class-E - basic unit stress 1,133 p.s.i.
Structural Steel - ASTM A36 - basic unit stress 20,000 p.s.i. (ASTM A7 and A373 steel not permitted)
Reinforcing Steel - ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 p.s.i. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 p.s.i.

GENERAL PLAN

		ESTIMATED QUANTITIES				LEFT BRIDGE				RIGHT BRIDGE					
Item	Total	Unit	Description	Superstr.	Abut.	Piers	General	Superstr.	Abut.	Piers	General	Superstr.	Abut.	Piers	General
E-2	Lump Sum	Lump Sum	Coffer dams, Cribbs and Sheeting				Lump Sum								
E-2	931	Cu.Yd.	Unclassified Excavation		161	330			143	297					
S-1	829	Cu.Yd.	Class "C" Concrete, Superstructure	448				381							
S-1	180	Cu.Yd.	Class "C" Concrete, Pier Caps & Columns			91	90			3290					
S-1	244	Cu.Yd.	Class "E" Concrete, Pier Footings				122			122					
S-1	174	Cu.Yd.	Class "E" Concrete, Abutments			92				82					
S-4	291,246	Lbs.	Reinforcing Steel	107,361	8868	46,634		90,038	7870	30,475					
S-14	669.66	Lin.Ft.	Railing (Type "A", Aluminum Rail and Supports, Concrete Parapet)	294.83	40.00			294.83	40.00						
S-16	Lump Sum	Lump Sum	First Test Pile				Lump Sum								
S-18	3843	Lin.Ft.	10 BP 42# Steel H Piles		528	1590			477	1248					
* S-25			Electric Lighting System, complete												
S-29	11	Each	Scuppers (4" x 18" Long C.I. or W.I. pipe)	6					5						
S-29	27	Cu.Yd.	Porous Backfill		15					12					
I-10	757	Sq.Yd.	Concrete Slope Protection				401				356				
S-101	829	Each	Water-reducing, Set-retarding Admixture	448				381							

* See General Lighting Summary, Sheet No. 199 for detailed description, unit and quantity.



ELEVATION

Station	DECK ELEVATIONS									
	A	B	C	J	E	F	G	H	D	
518+70	1023.212	1023.283	1023.70	1023.51	1023.434	1023.51	1023.706	1023.352	1023.43	
+80	.122	.792	.61	.42	.346	.42	.612	.260	.34	
+90	.020	.092	.51	.32	.257	.32	.44	.180	.245	
519+00	1022.927	1022.920	.41	.22	.169	.22	.362	1022.109	.145	
+10	.823	1022.951	.31	.12	.079	.12	.249	.028	.04	
+20	.720	.900	.20	.01	1022.989	.01	.102	1022.947	1022.934	
+30	.614	.99.850	.09	1022.90	.879	1022.90	1022.973	.856	.823	
+40	1022.619	1022.79	1022.98	.79	.779	.79	.887	.764	.71	
+50	.584	.739	.86	.67	.679	.67	.695	.662	.5960	
+60	.48	.660	.74	.55	.568	.55	.550	.534	.478	
+70	.412	.587	.62	.43	.446	.43	.405	1021.339	.35	
+80	.376	.557	.49	.30	.384	.30	.240	.1384	.22	
+90	.369	.536	.36	.17	.242	.17	.094	1021.336	.09	
520+00	.336	.515	.23	.04	.0810	.04	1021.330	.725	1021.96	
+10	.305	.48	.09	1021.90	1021.937	1021.90	.772	.516	.82	
+20	.43	.254	.43	1021.976	.76	.824	.76	.38	.6035	.67
+30	1022.13	1022.365	1021.854	1021.61	1021.697	1021.61	1021.43	1021.68	1021.52	

PROCEDURE: The embankment shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade for a distance of 200 feet back of the abutments, after which excavation shall be made for the abutment and piles driven.

FIRST TEST PILE:

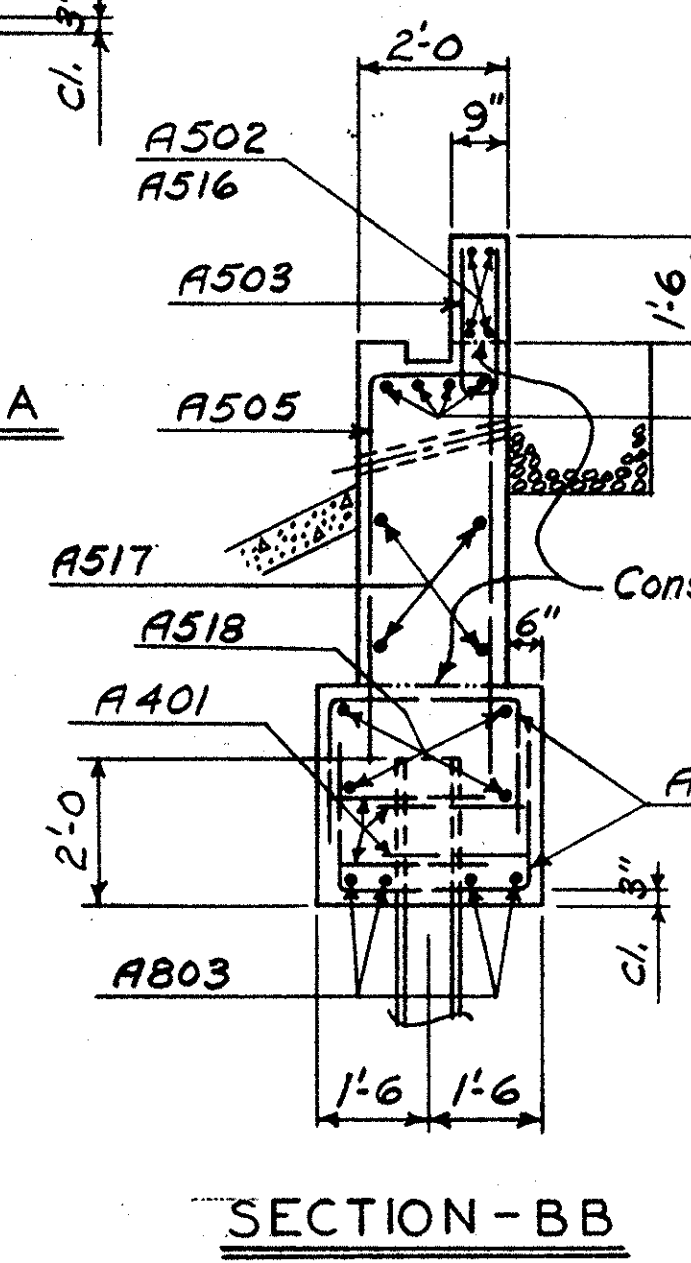
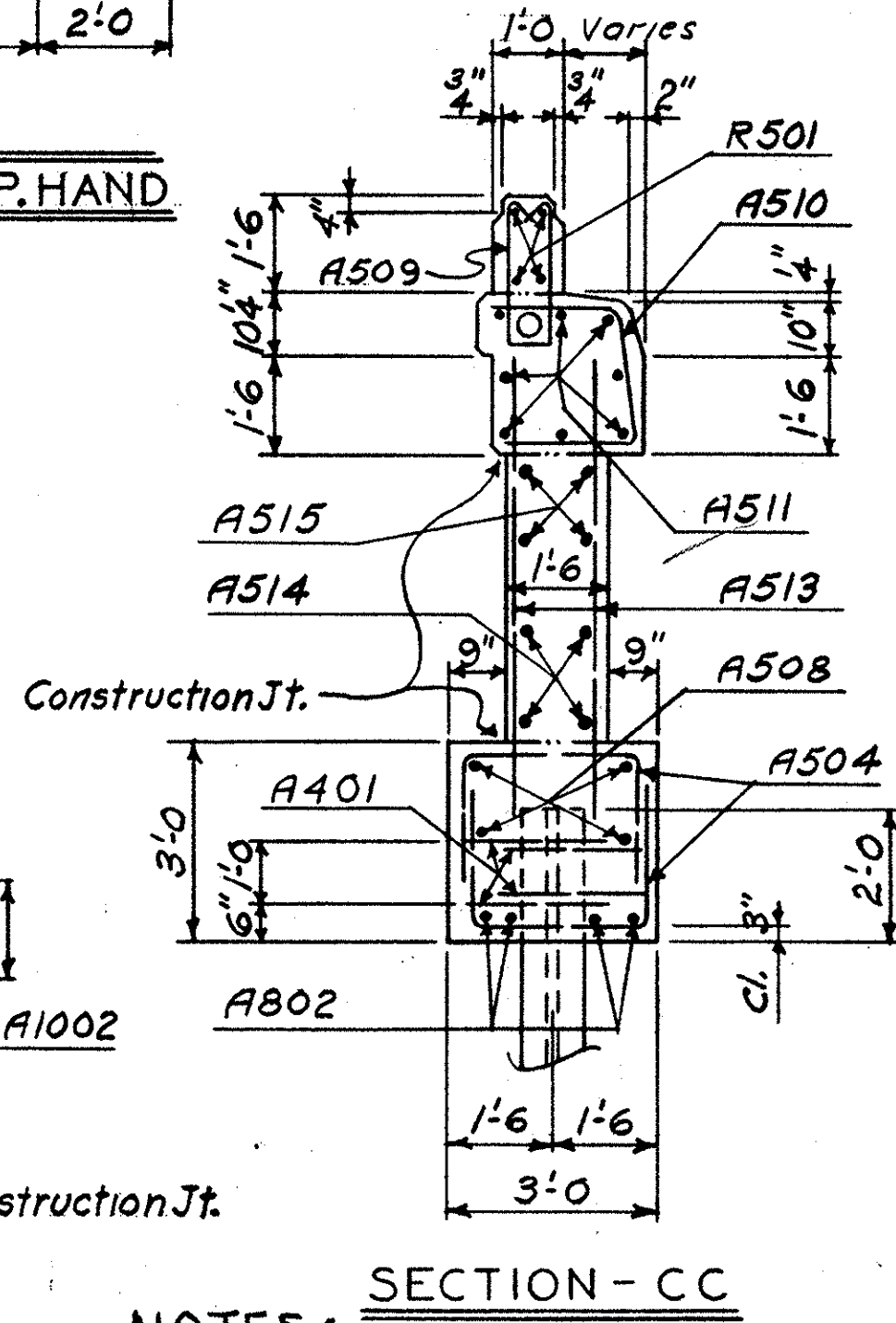
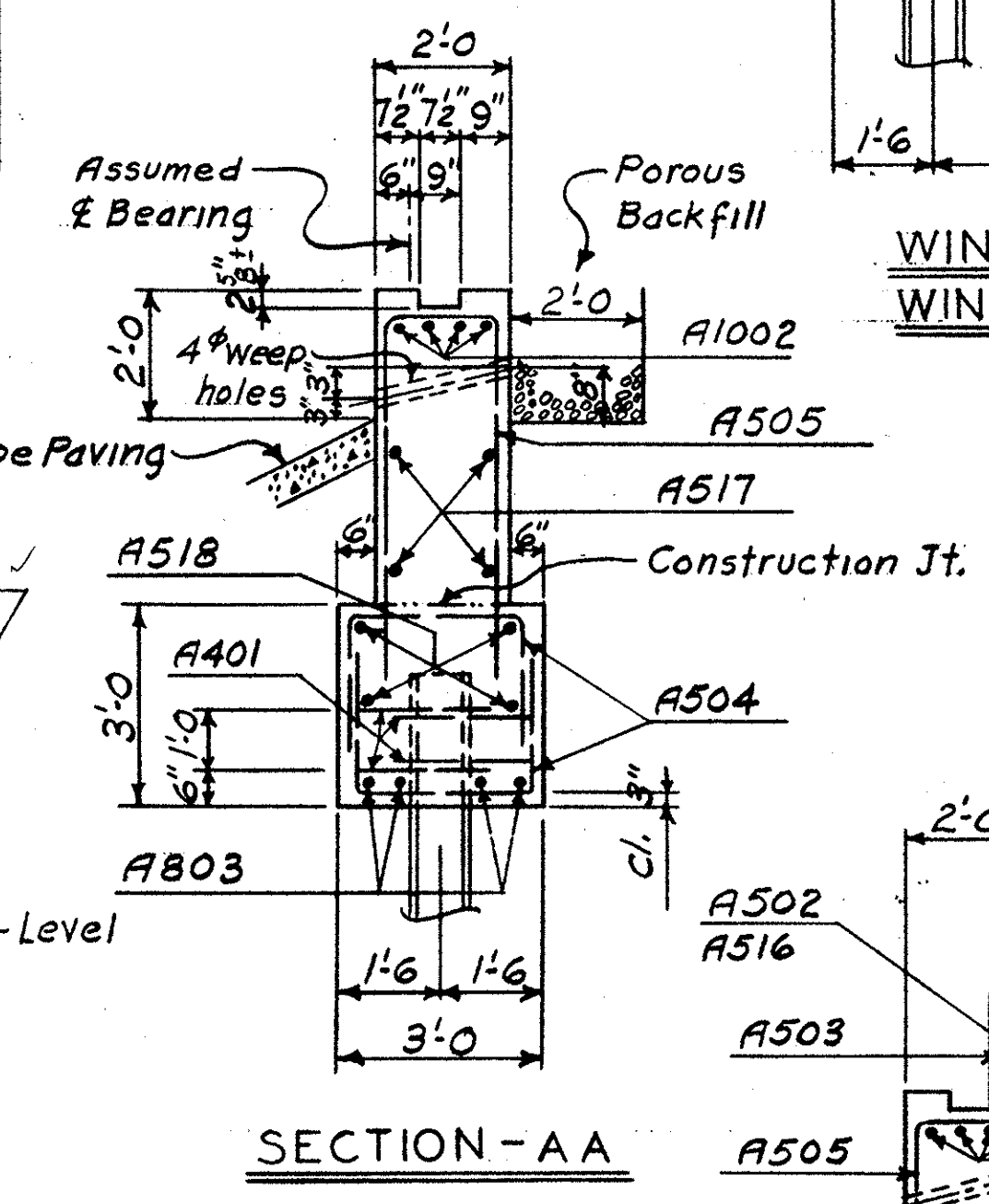
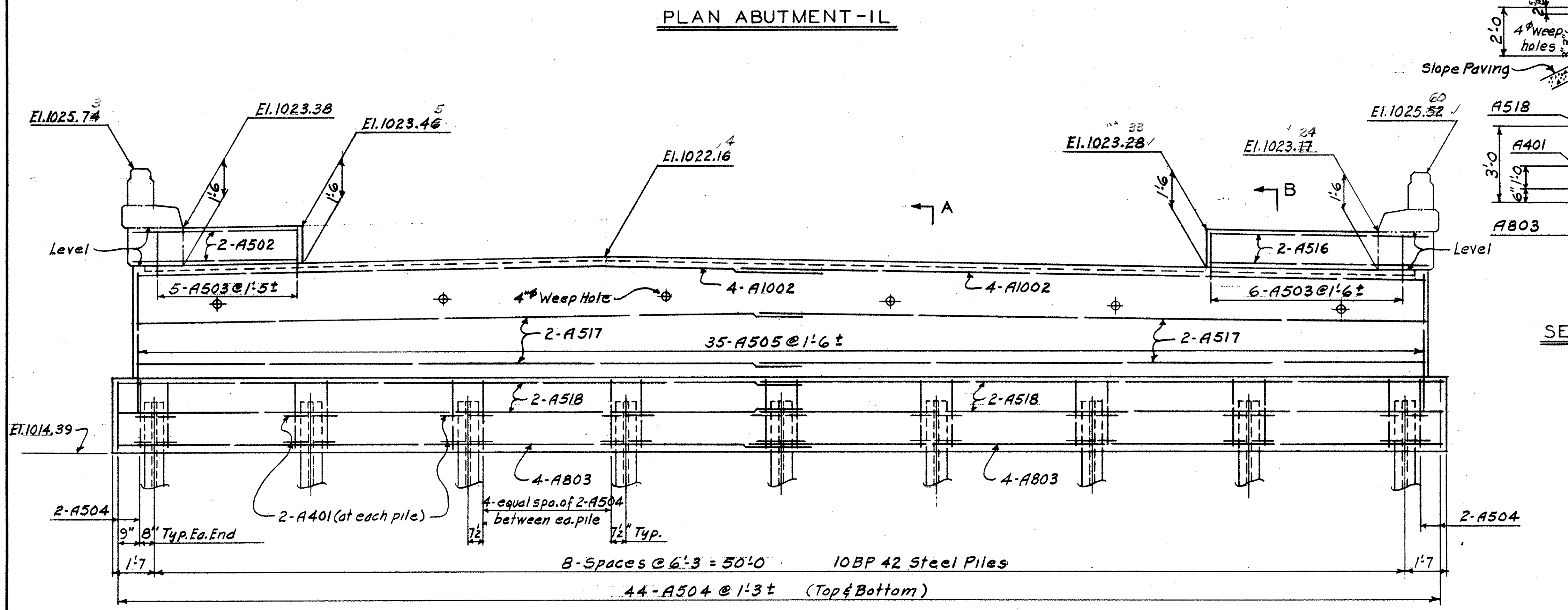
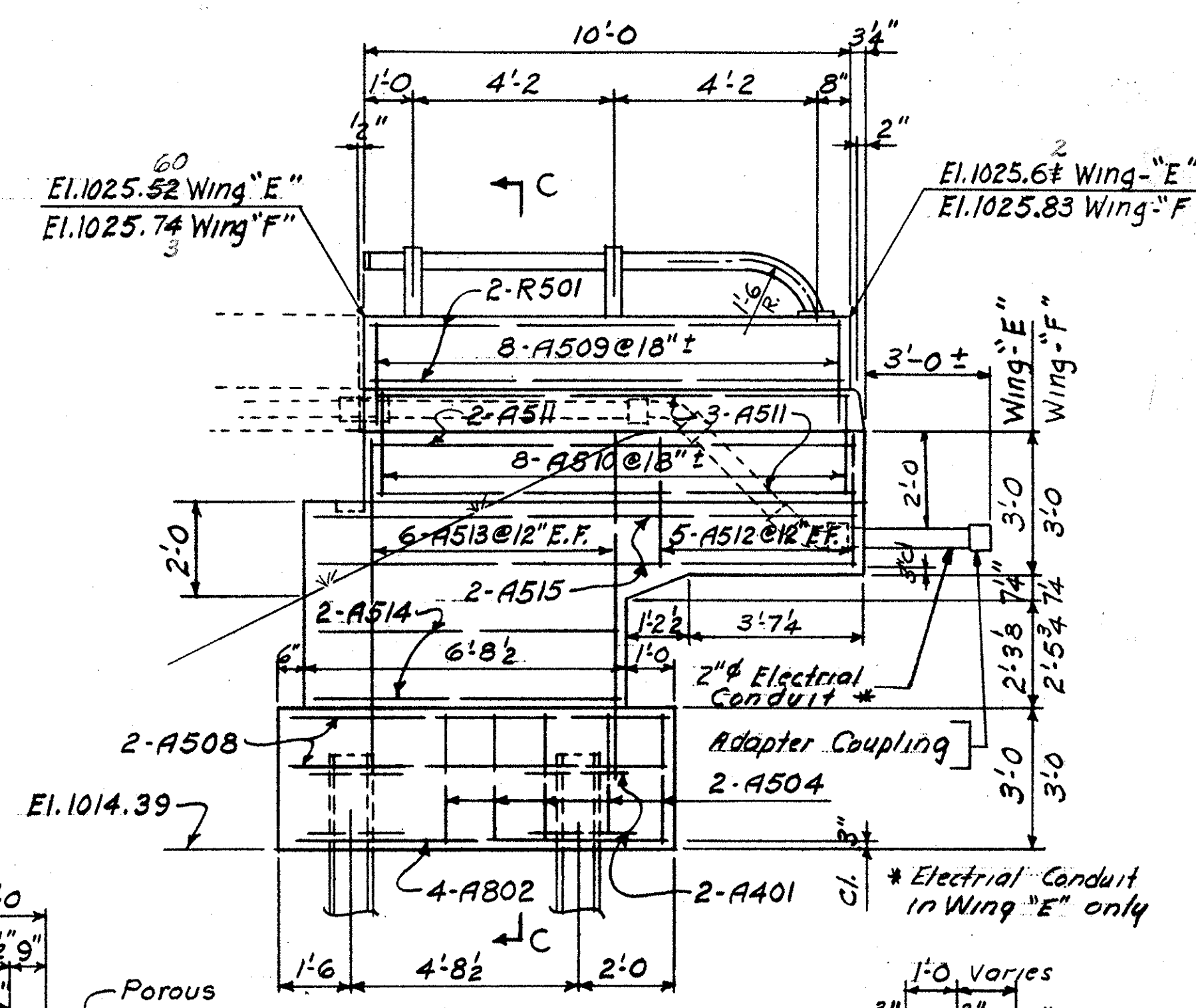
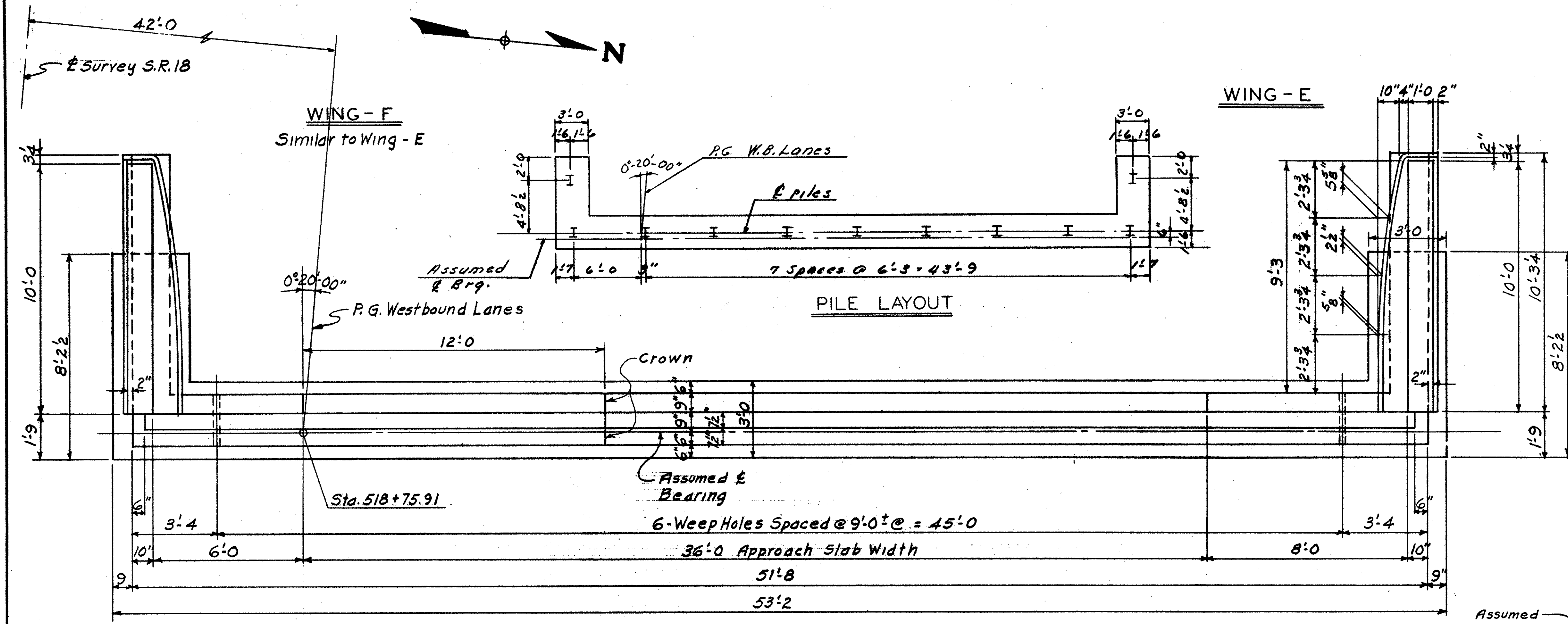
Payment will be made for only one first test pile. It may be driven for either the Right or the Left Bridge.

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

GENERAL PLAN & ELEVATION
BRIDGE NO. MAH-18-1552R/L
OVER MERIDIAN ROAD
Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	LPD		S.J.F.	L.G.H.	2-1-66
			P.A.	10-15-63	

MAHONING COUNTY
MAH-18-15.50

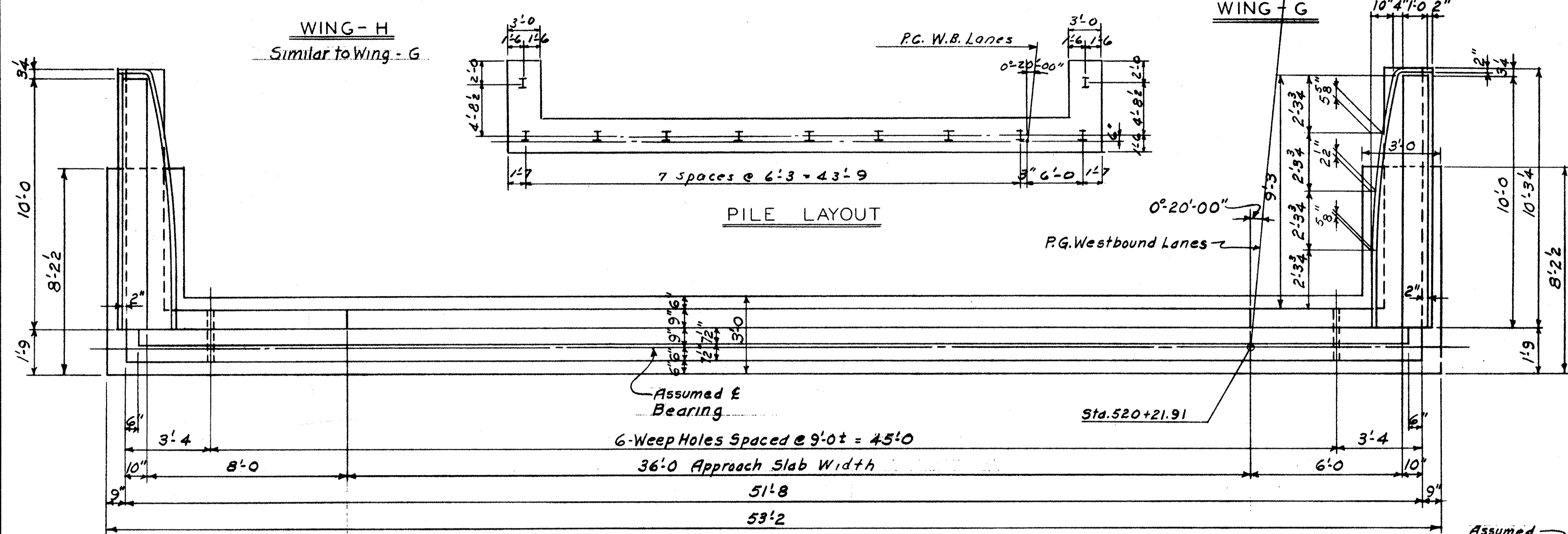


NOTES:
• For Abutment Notes see Sheet No. 211

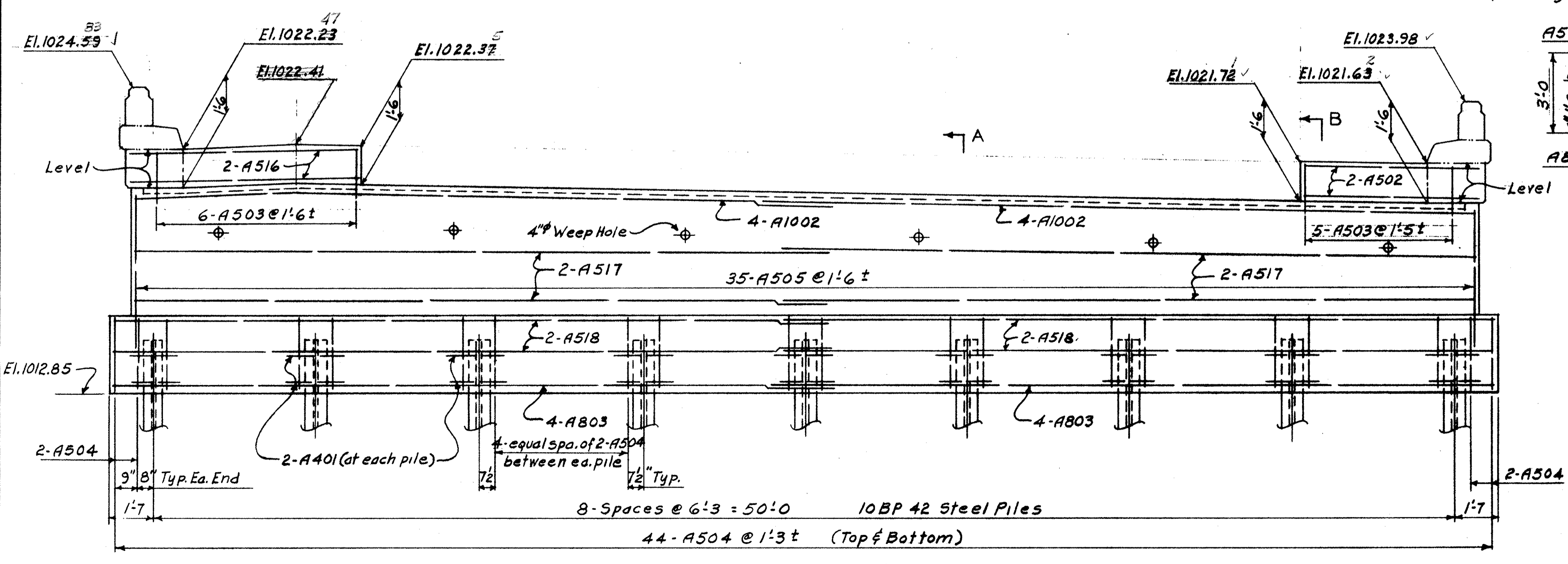
MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT NO.1 BRIDGE NO. MAH-18-1552L OVER MERIDIAN ROAD					
Sta. 518+75.40 To Sta. 520+22.90					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F	TP		S.J.F	L.G.H 10-15-63	2-1-66

Legend:
E.F. = Each Face

MAHONING COUNTY
MAH-18-15.50

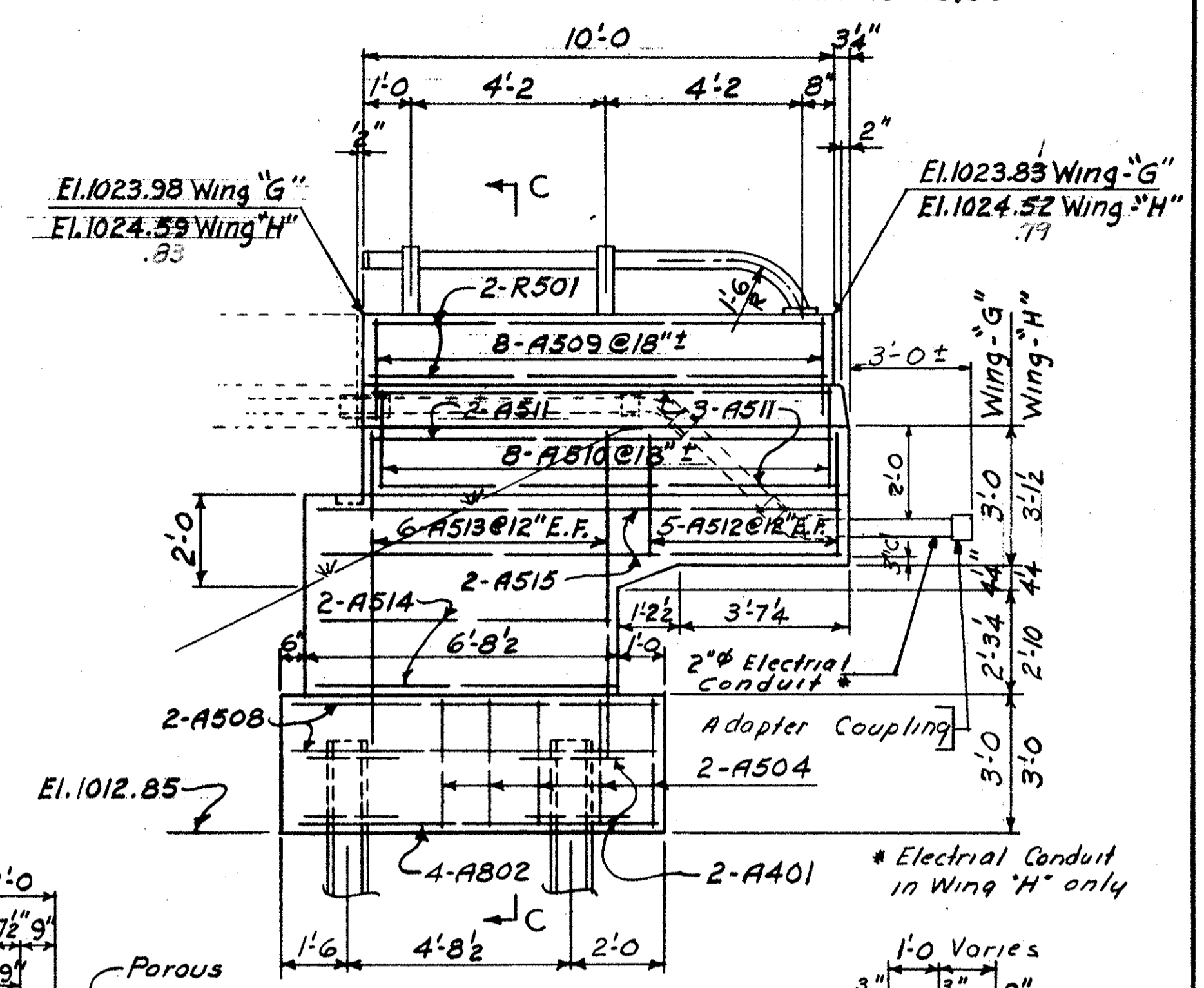


PLAN ABUTMENT -2L

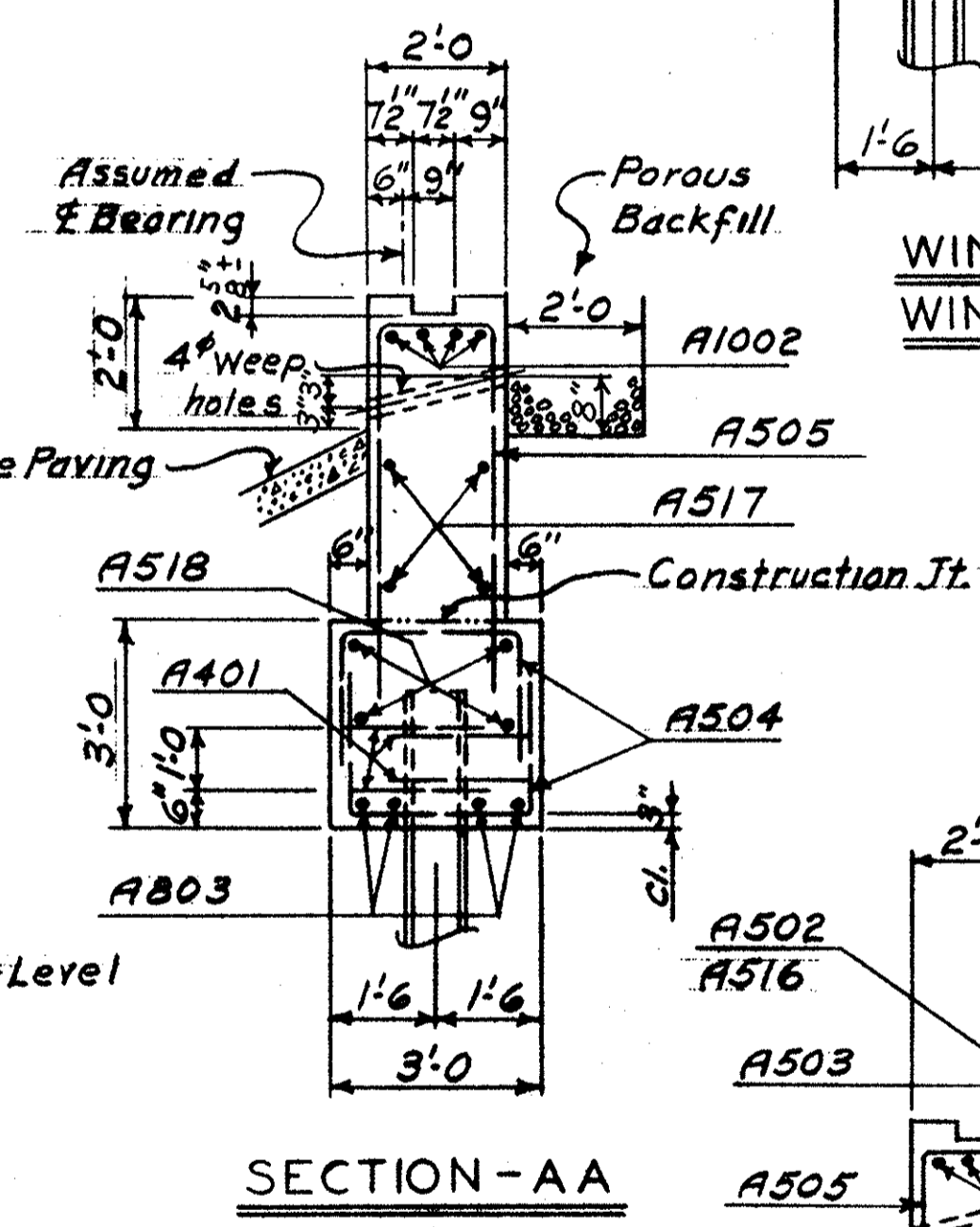


ELEVATION

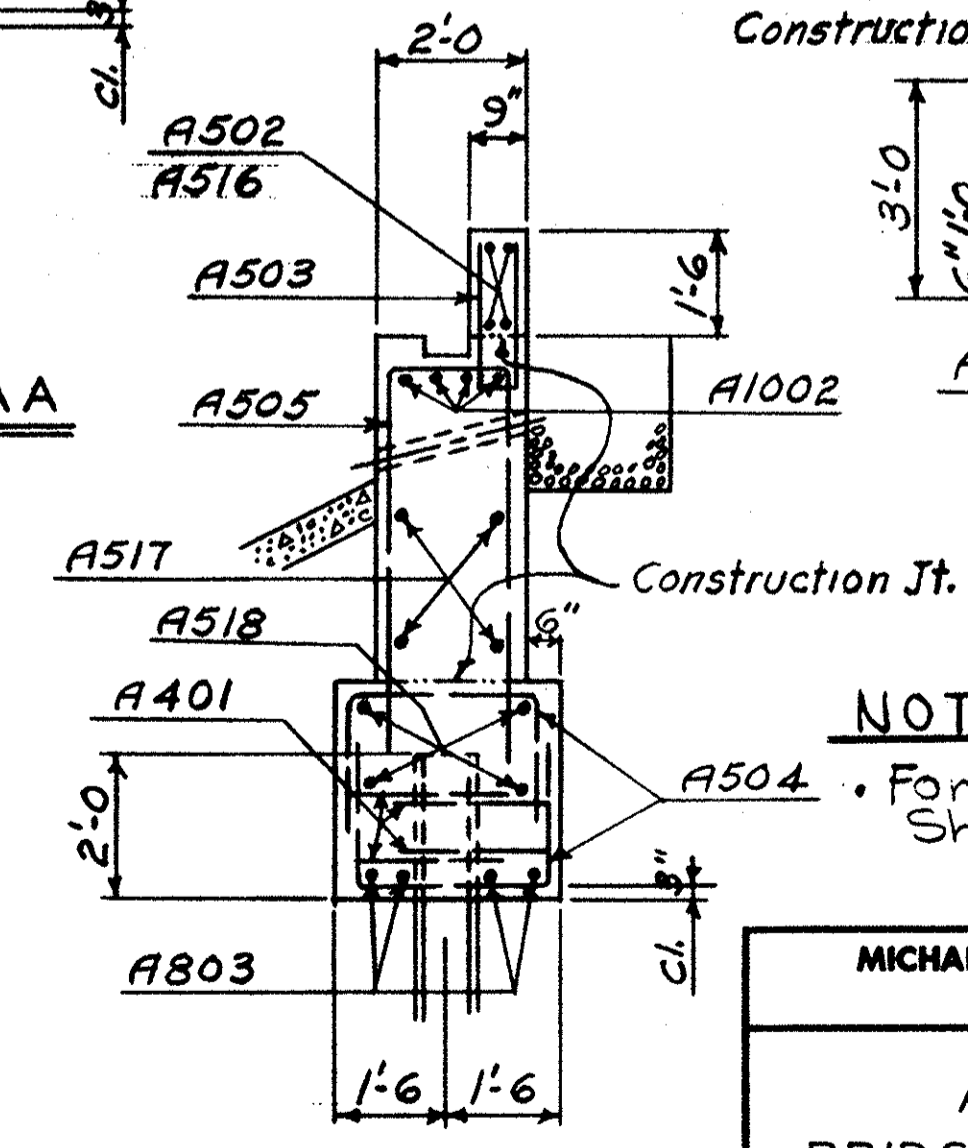
Legend
E.F. = Each Face



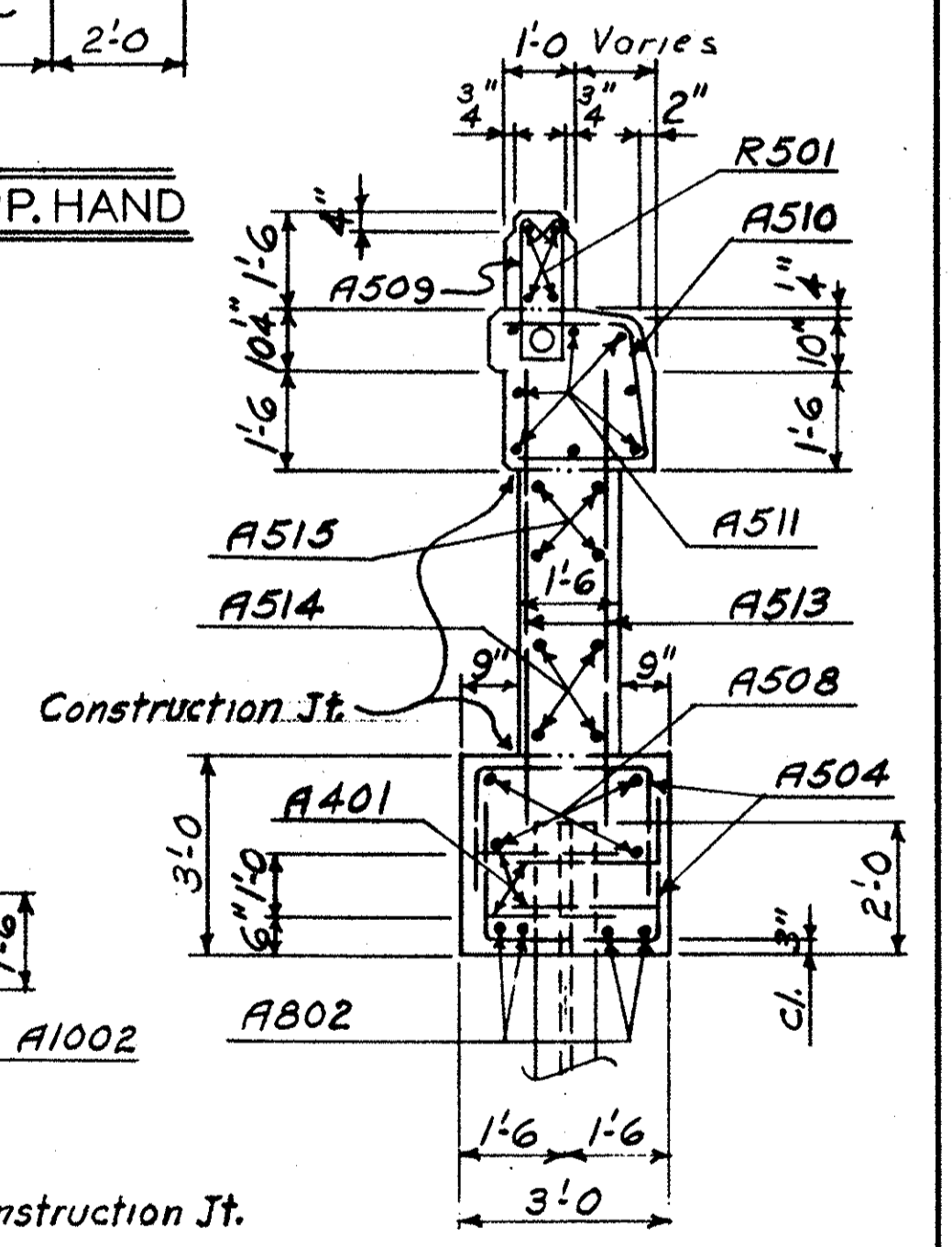
WING-G
WING-H OPP. HAND



SECTION-AA



SECTION-BB

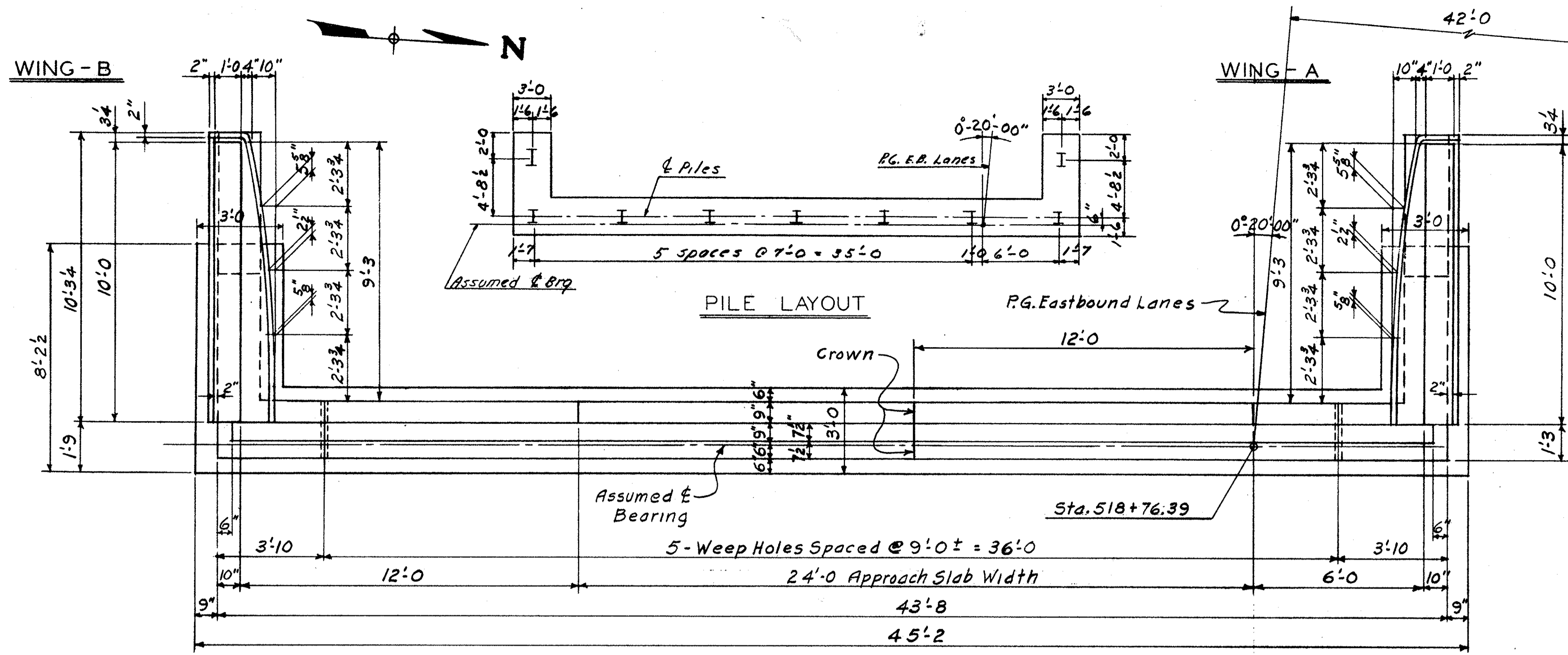


SECTION-CC

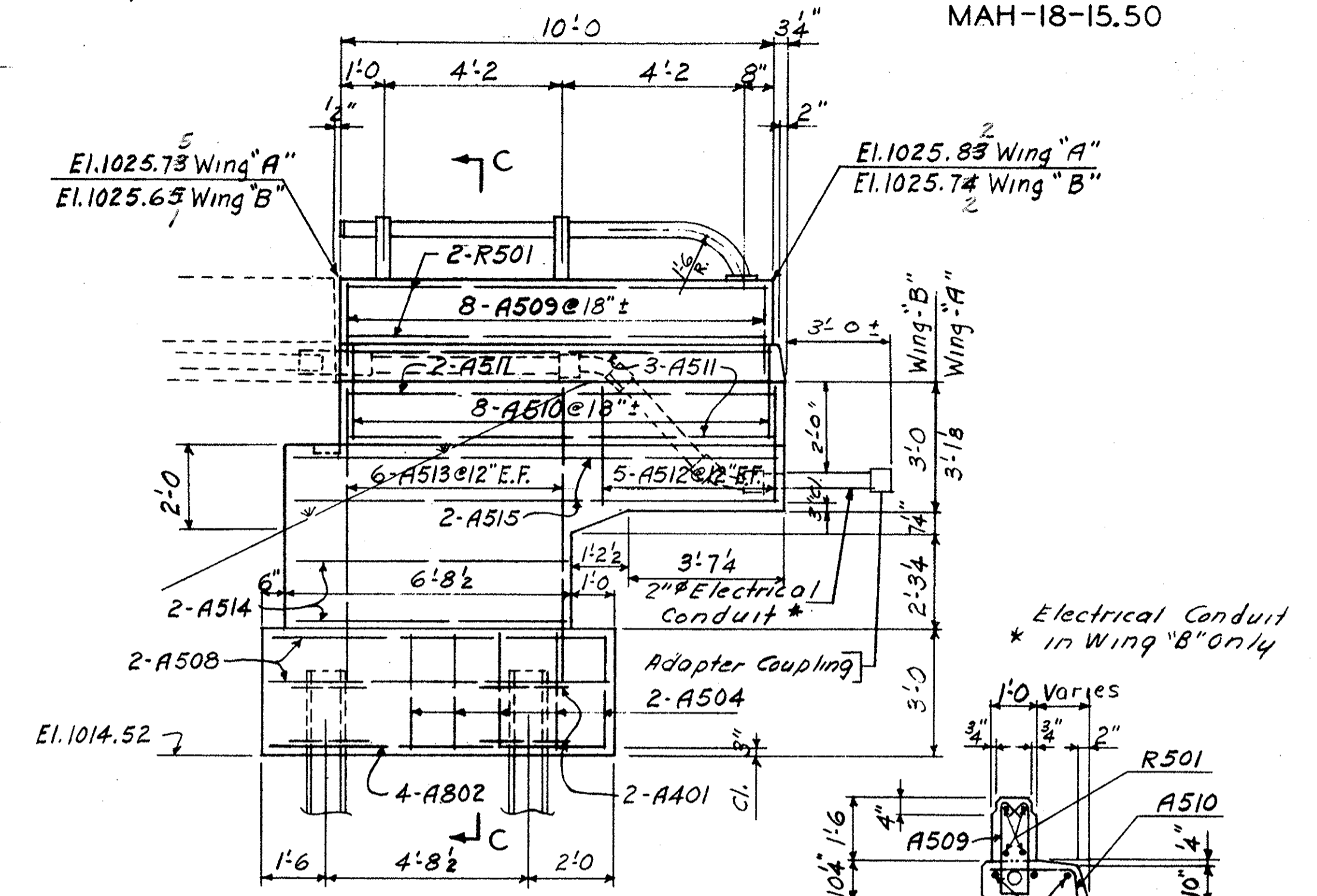
NOTES:
For Abutment Notes see Sheet No. 211

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT NO. 2 BRIDGE NO. MAH-18-1552L OVER MERIDIAN ROAD					
Sta. 518+75.40 To Sta. 520+22.90					
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S.J.F.	TP		S.J.F.	LGH 10-15-63	2-1-66

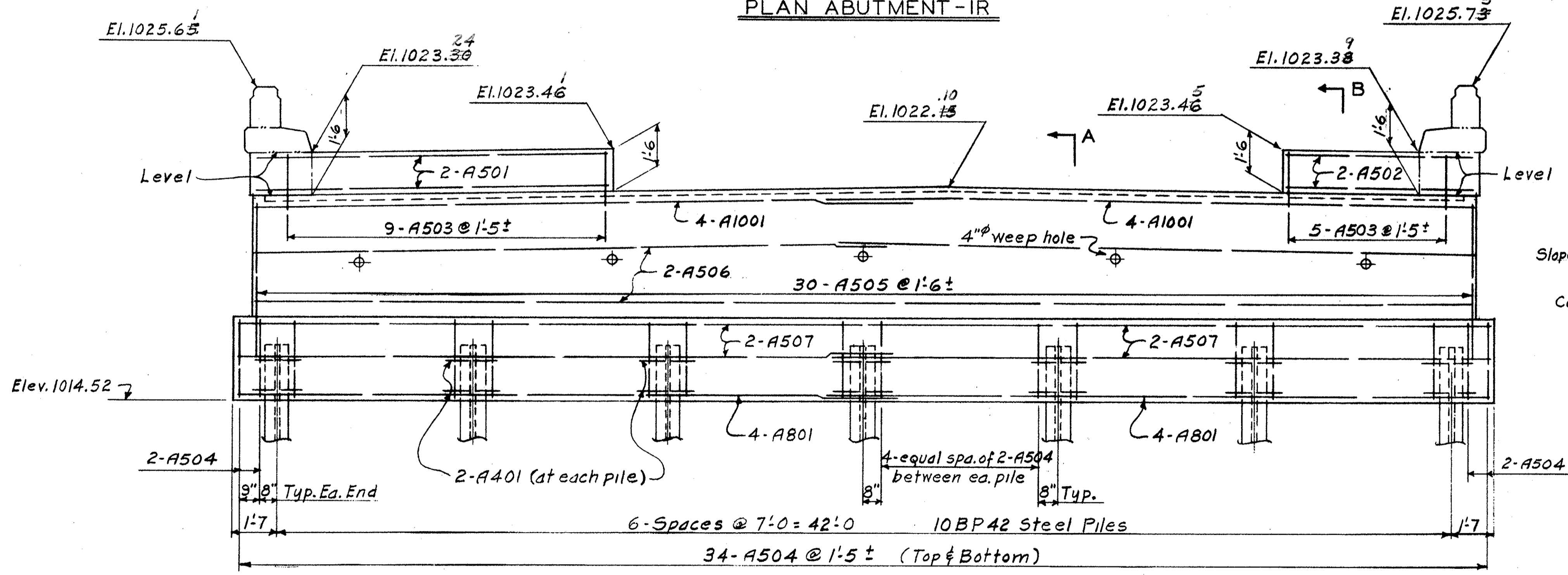
MAHONING COUNTY
MAH-18-15.50



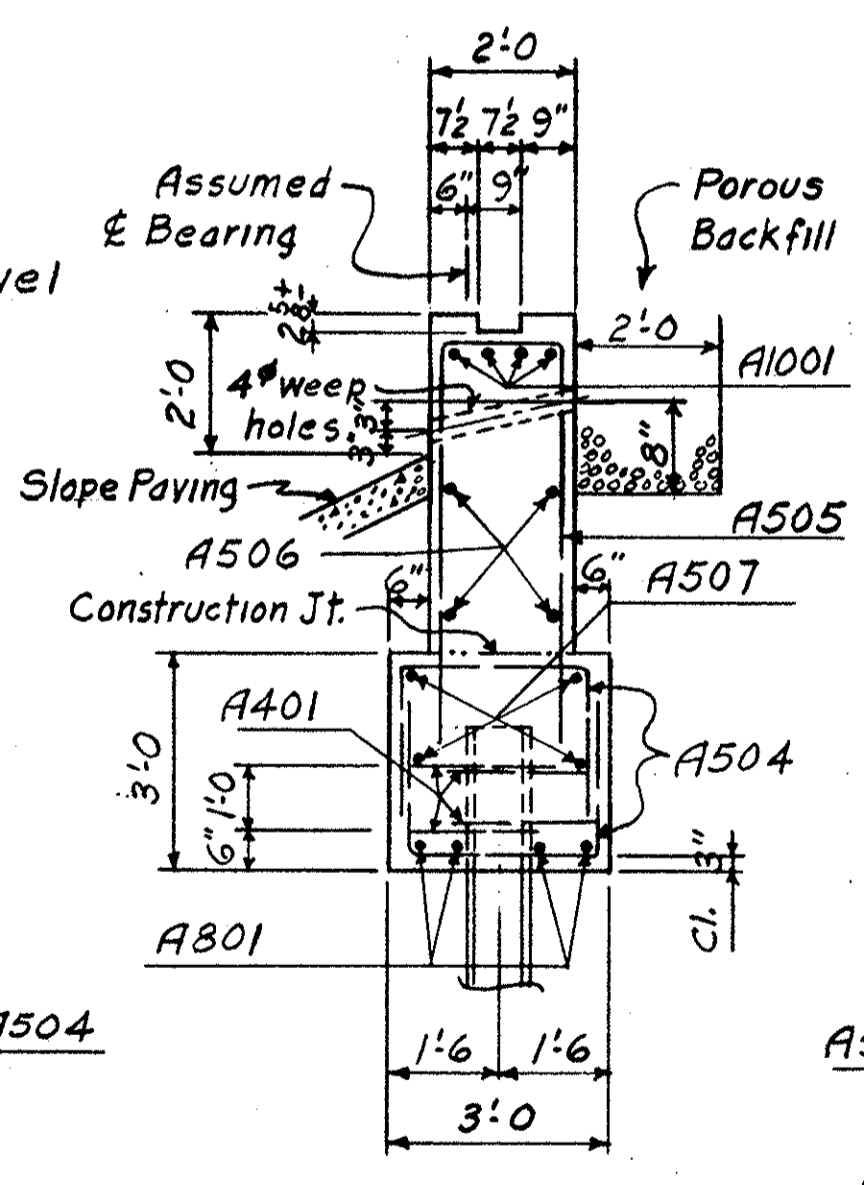
PLAN ABUTMENT-IR



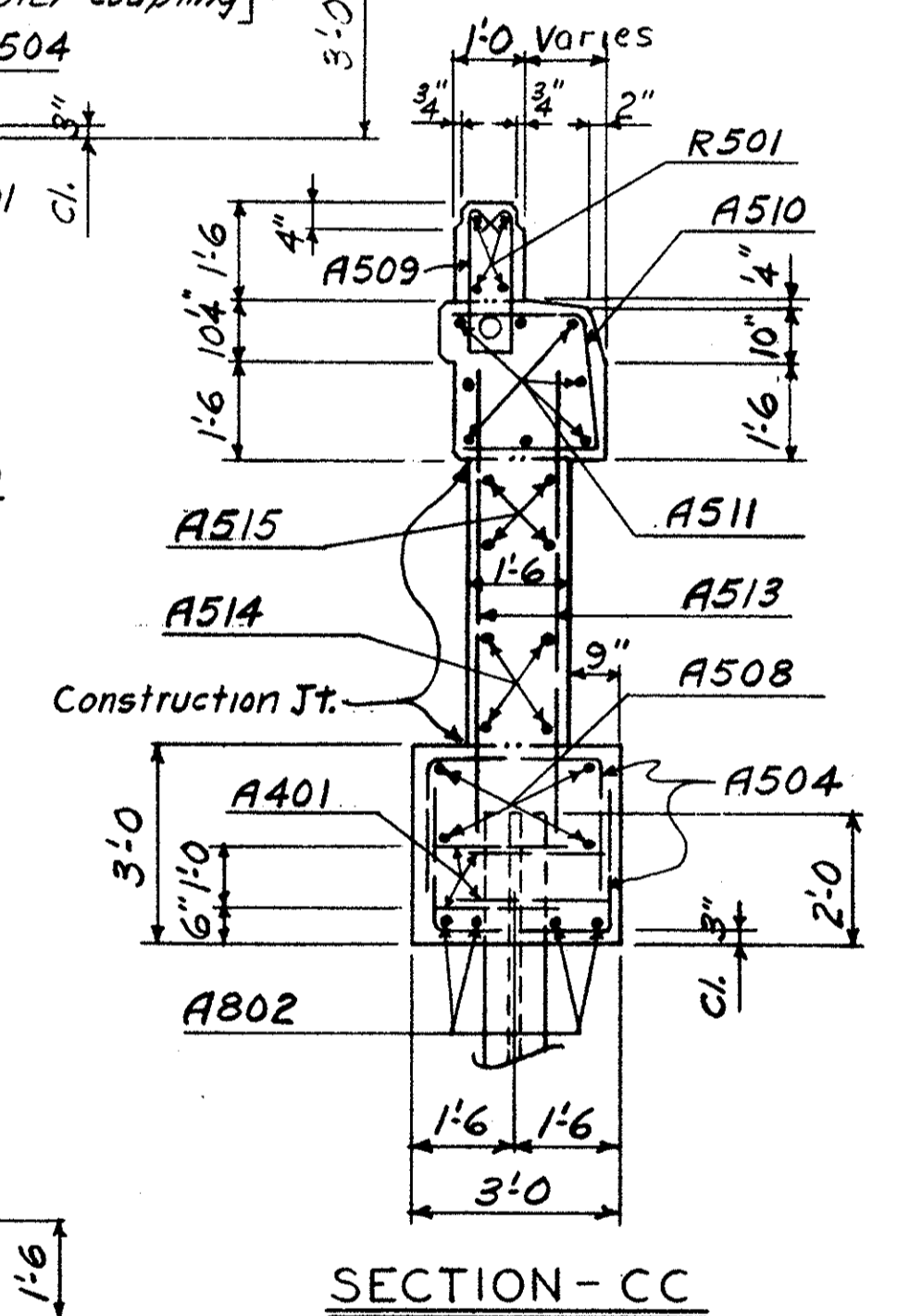
WING-A
WING-B OPP. HAND



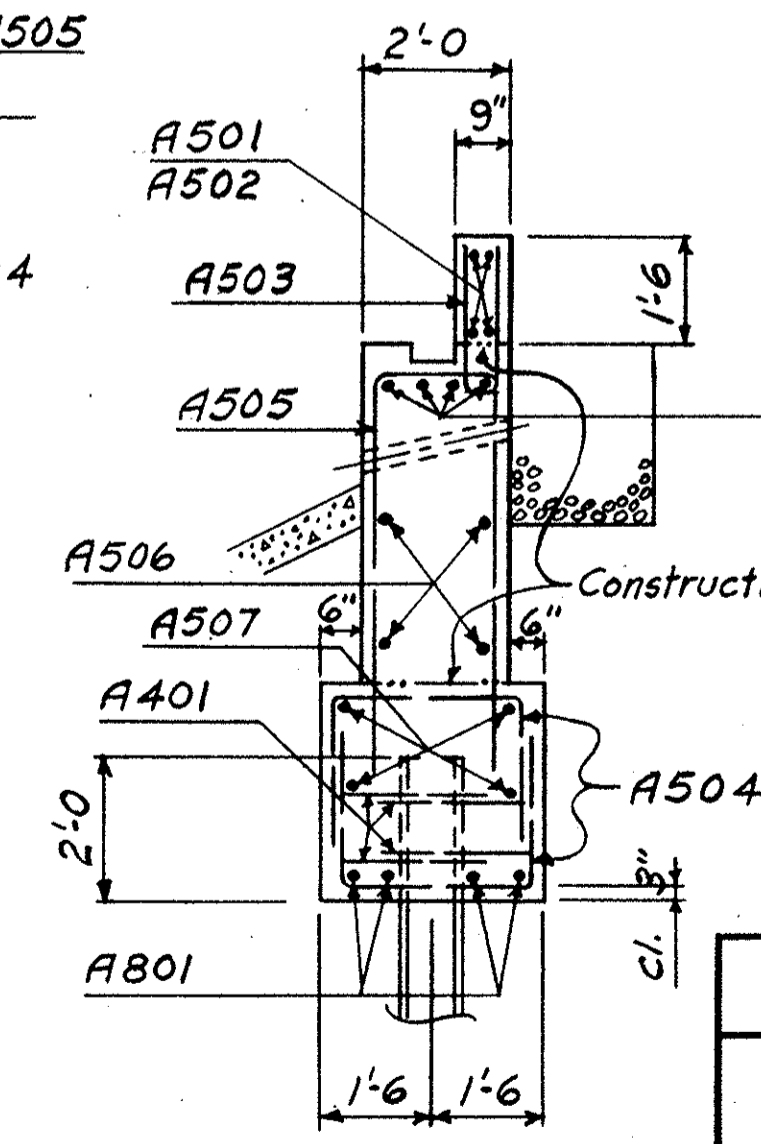
ELEVATION



SECTION-AA



SECTION-CC



SECTION-BB

Legend:
E.F. = Each Face

NOTES:

- POROUS BACKFILL, 2 Ft. thick, full length of abutment, shall extend upward to the approach slab and to the surface of the shoulders. Excavation therefor, in excess of that required for construction of the abutment, shall be considered as paid for in the bid price per cu. yd. paid for porous backfill.
- Concrete shall be class "E" except parapet concrete which shall be class "C"

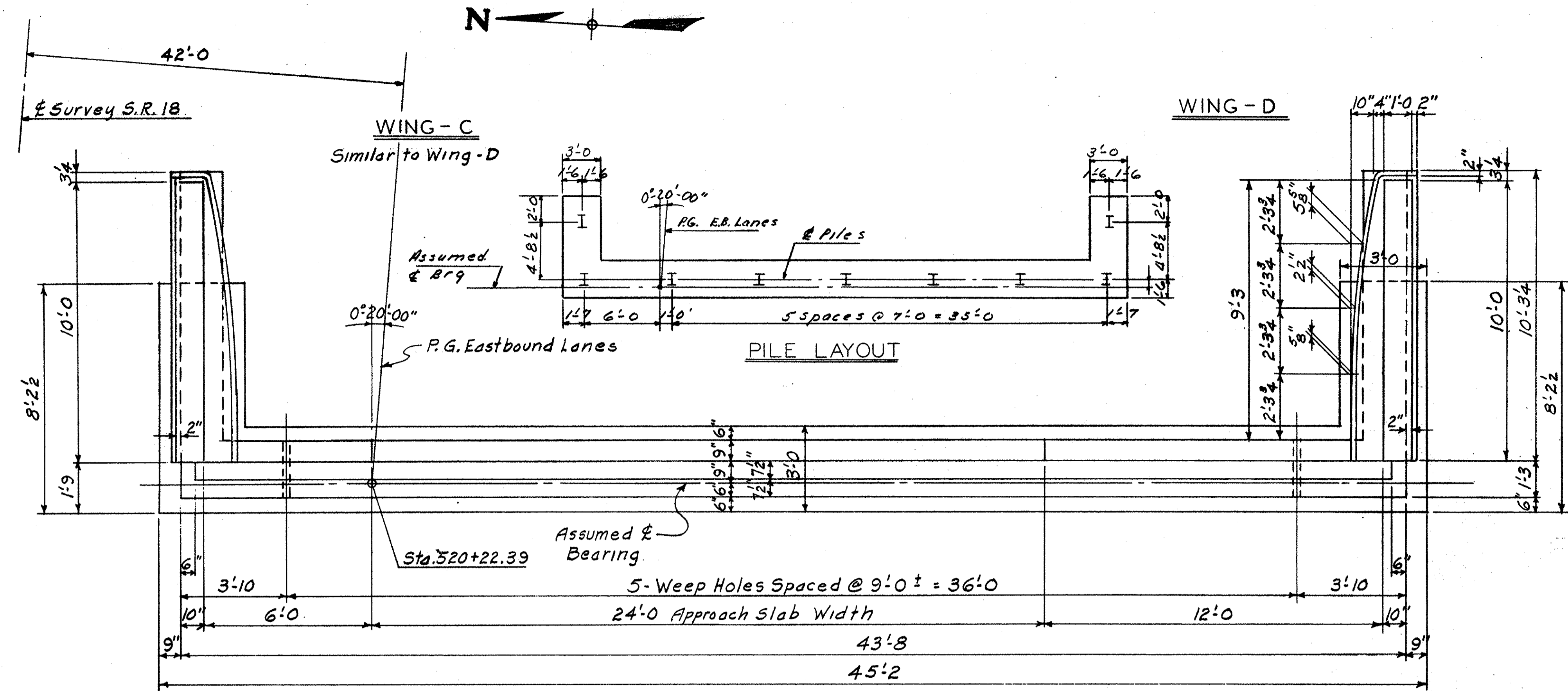
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

ABUTMENT NO.1
BRIDGE NO. MAH-18-1552R
OVER MERIDIAN ROAD

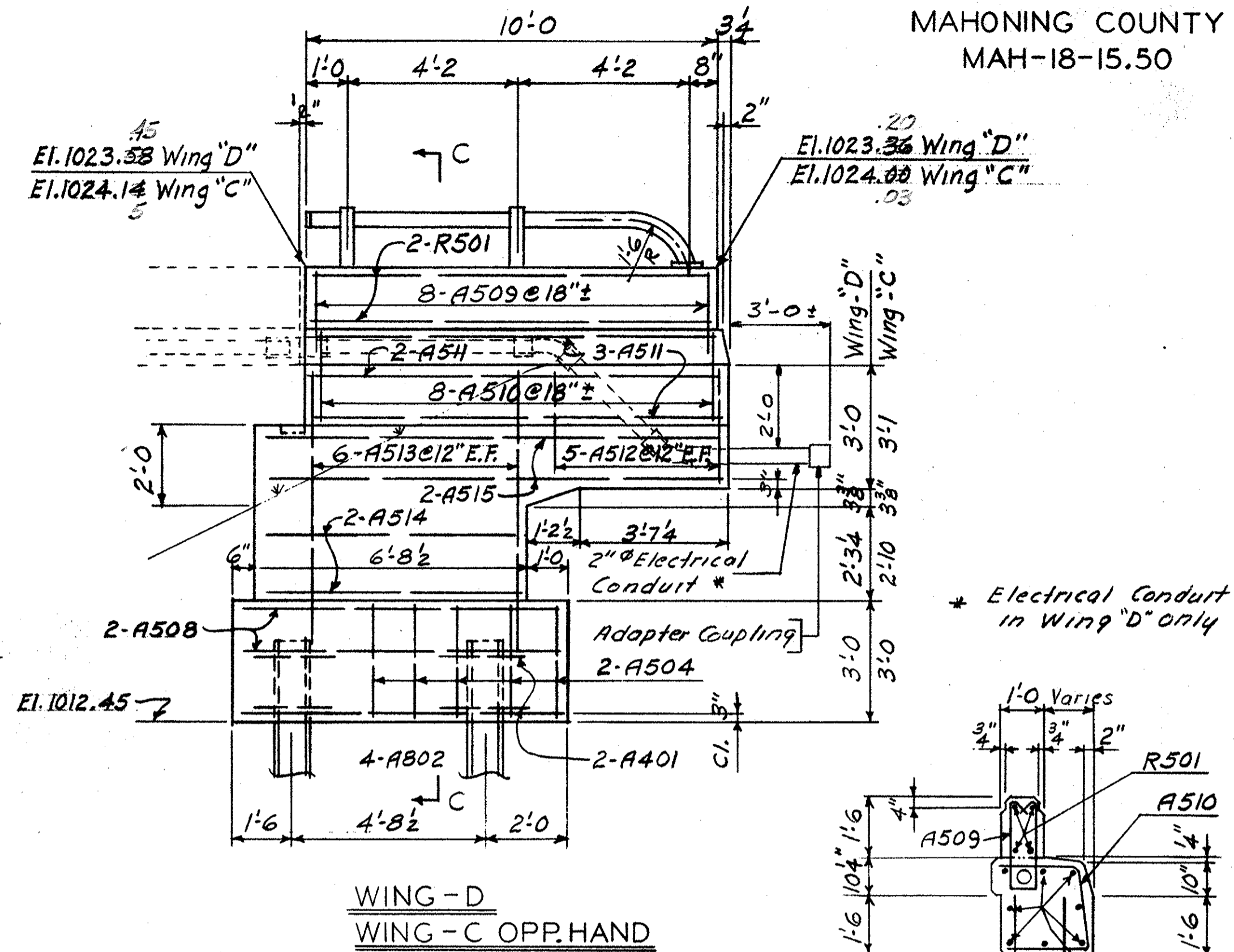
Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	TP		SJF	L.G.H. 10-15-63	2-1-66

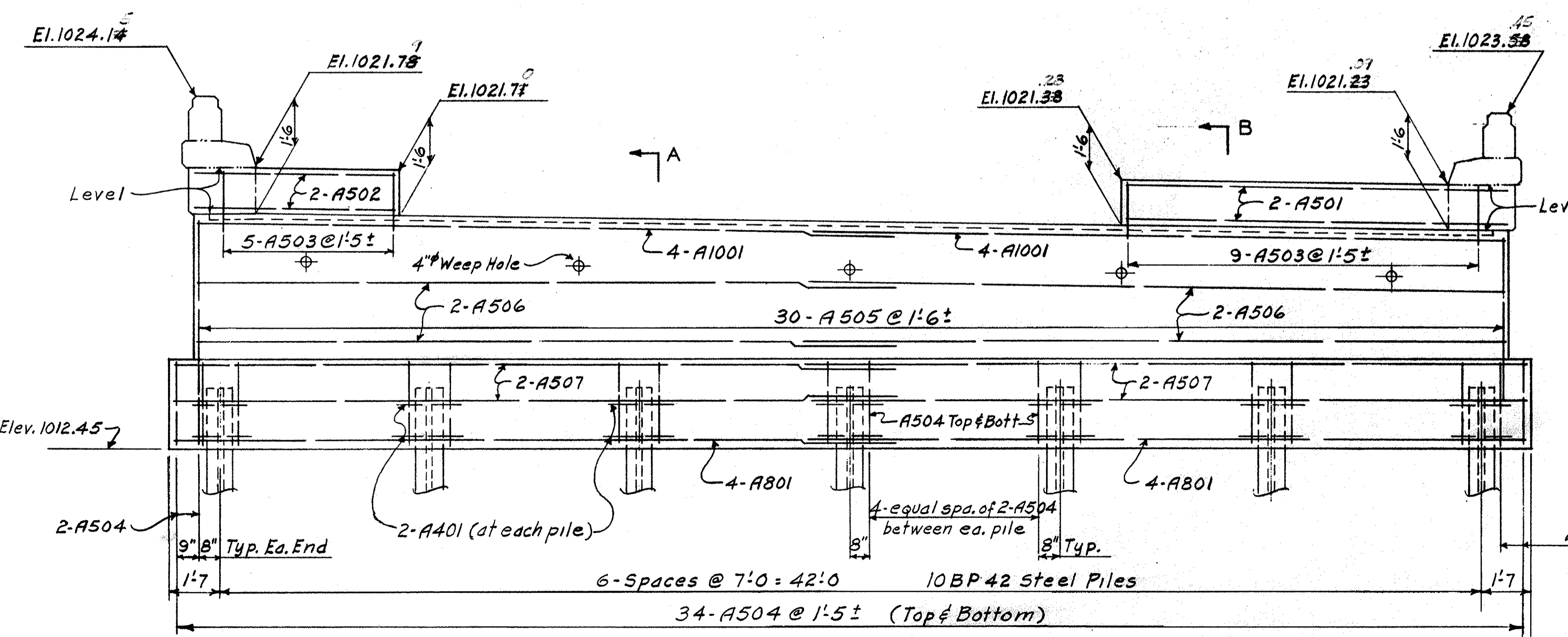
MAHONING COUNTY
MAH-18-15.50



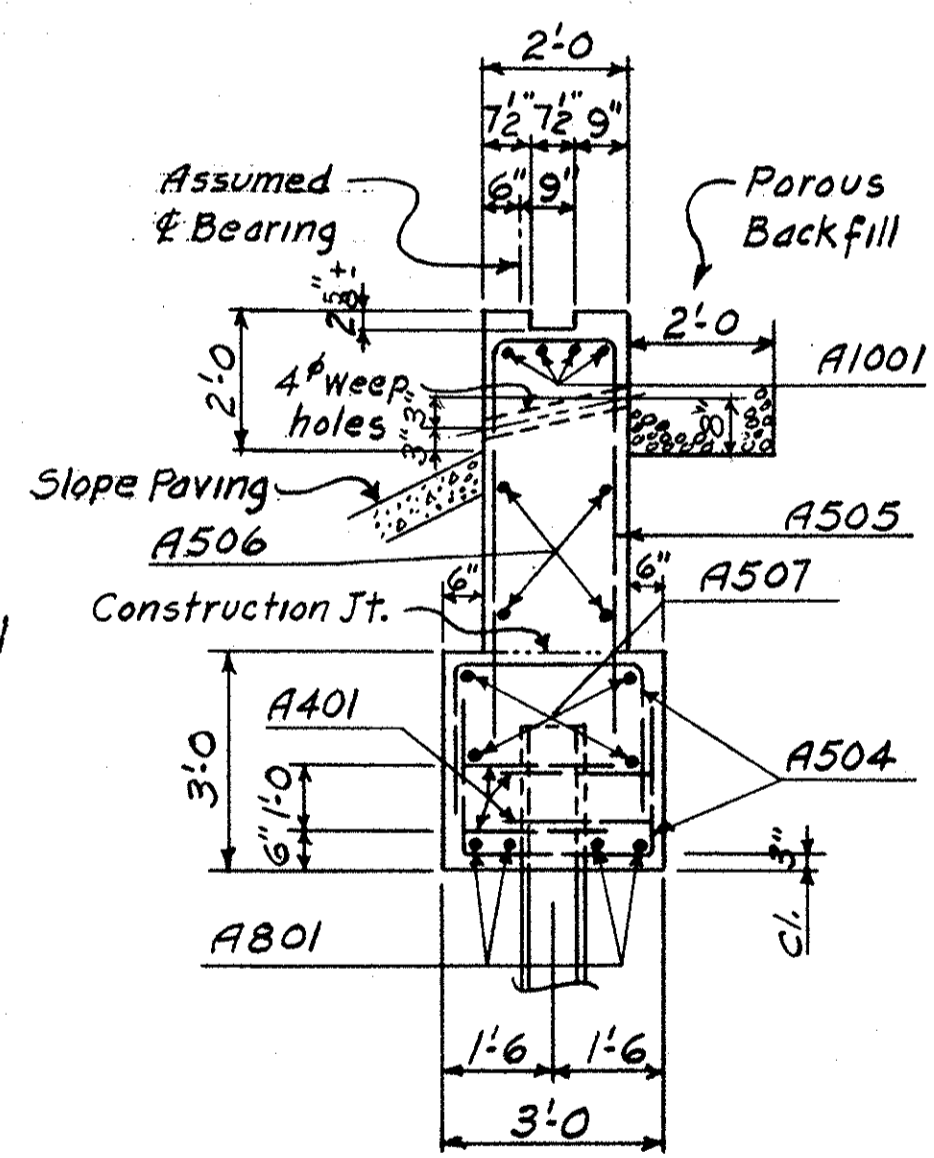
PLAN ABUTMENT - 2R



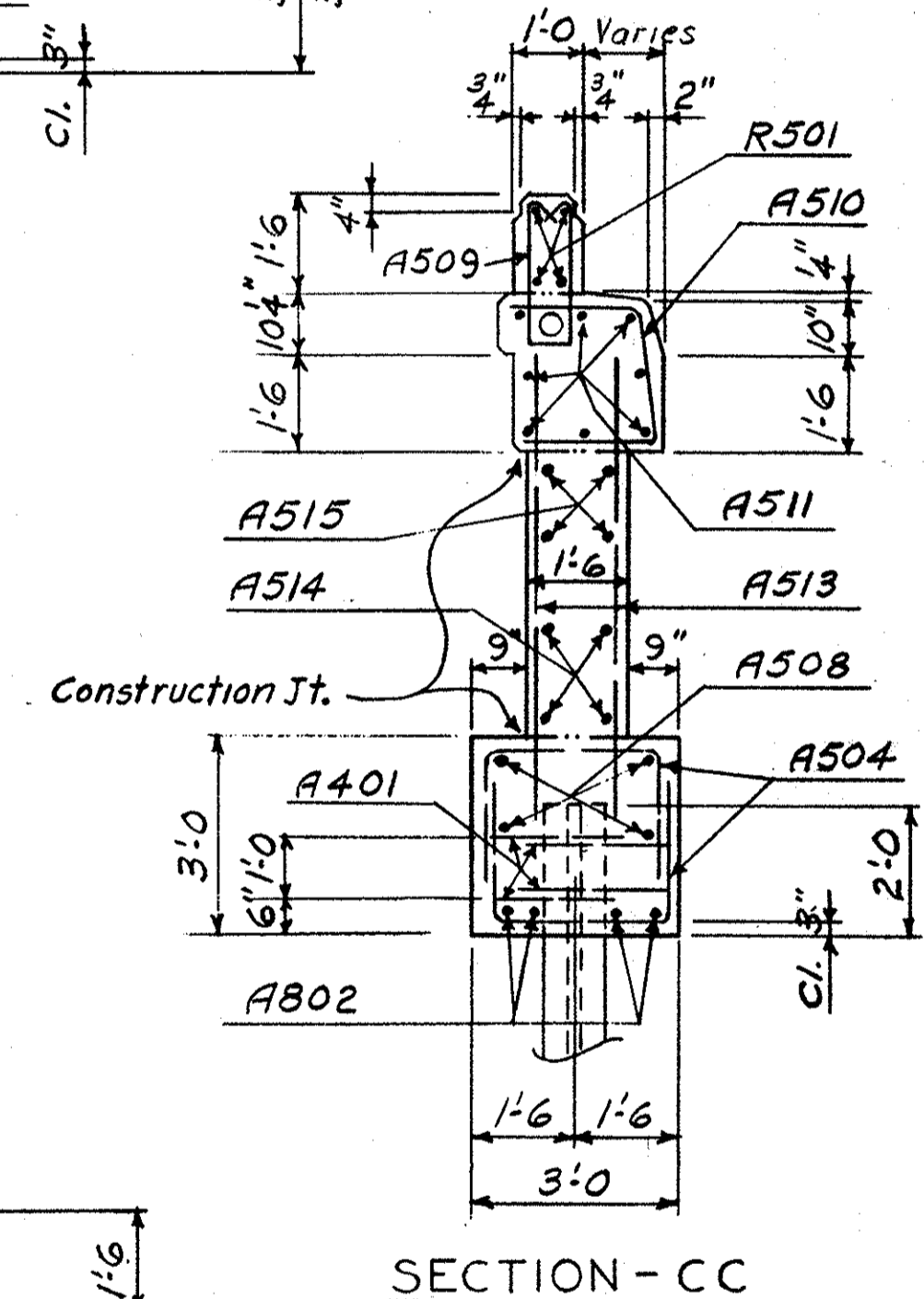
WING - D
WING - C OPP. HAND



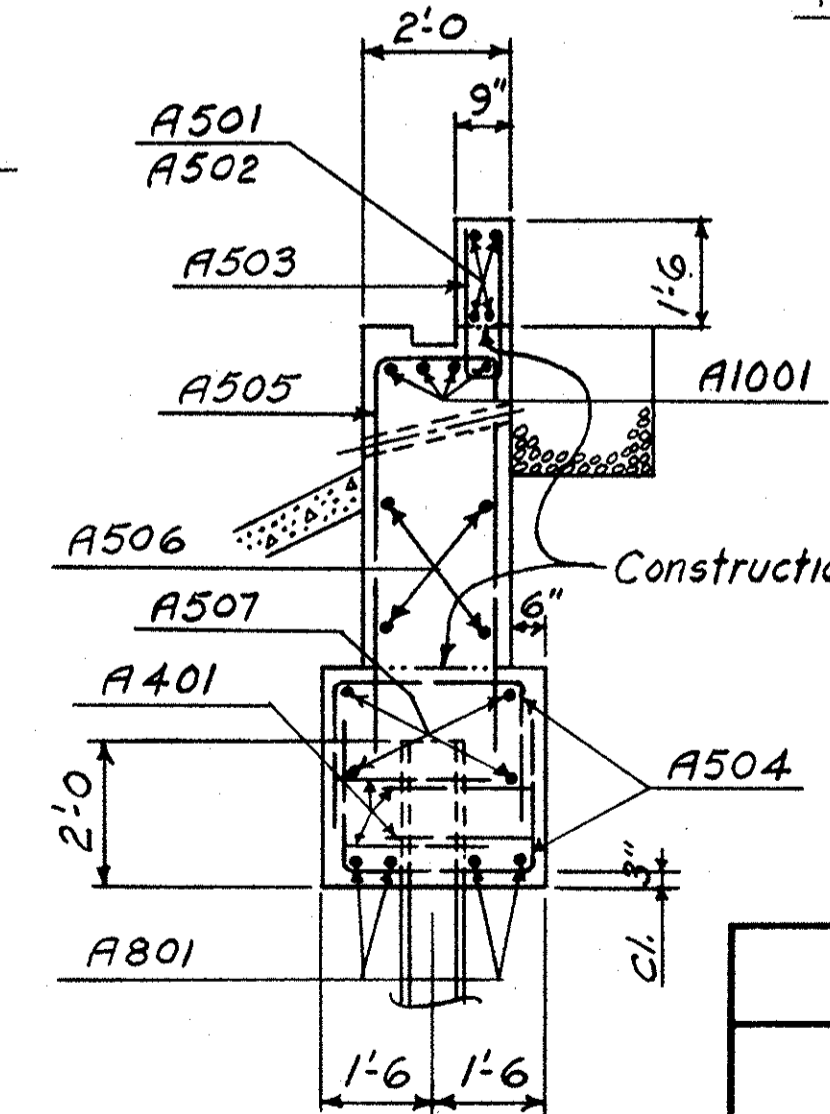
ELEVATION



SECTION - AA



SECTION - CC



SECTION - BB

Notes
* For Abutment notes see sht # 211

Legend:
E.F. = Each Face

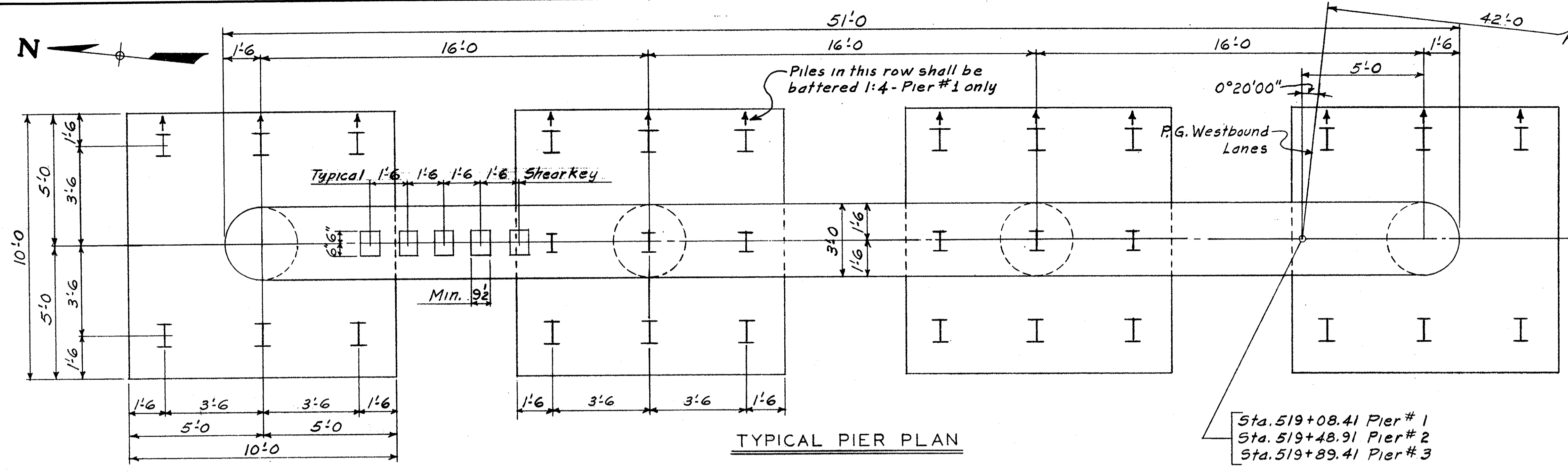
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

ABUTMENT NO. 2
BRIDGE NO. MAH-18-1552R
OVER MERIDIAN ROAD

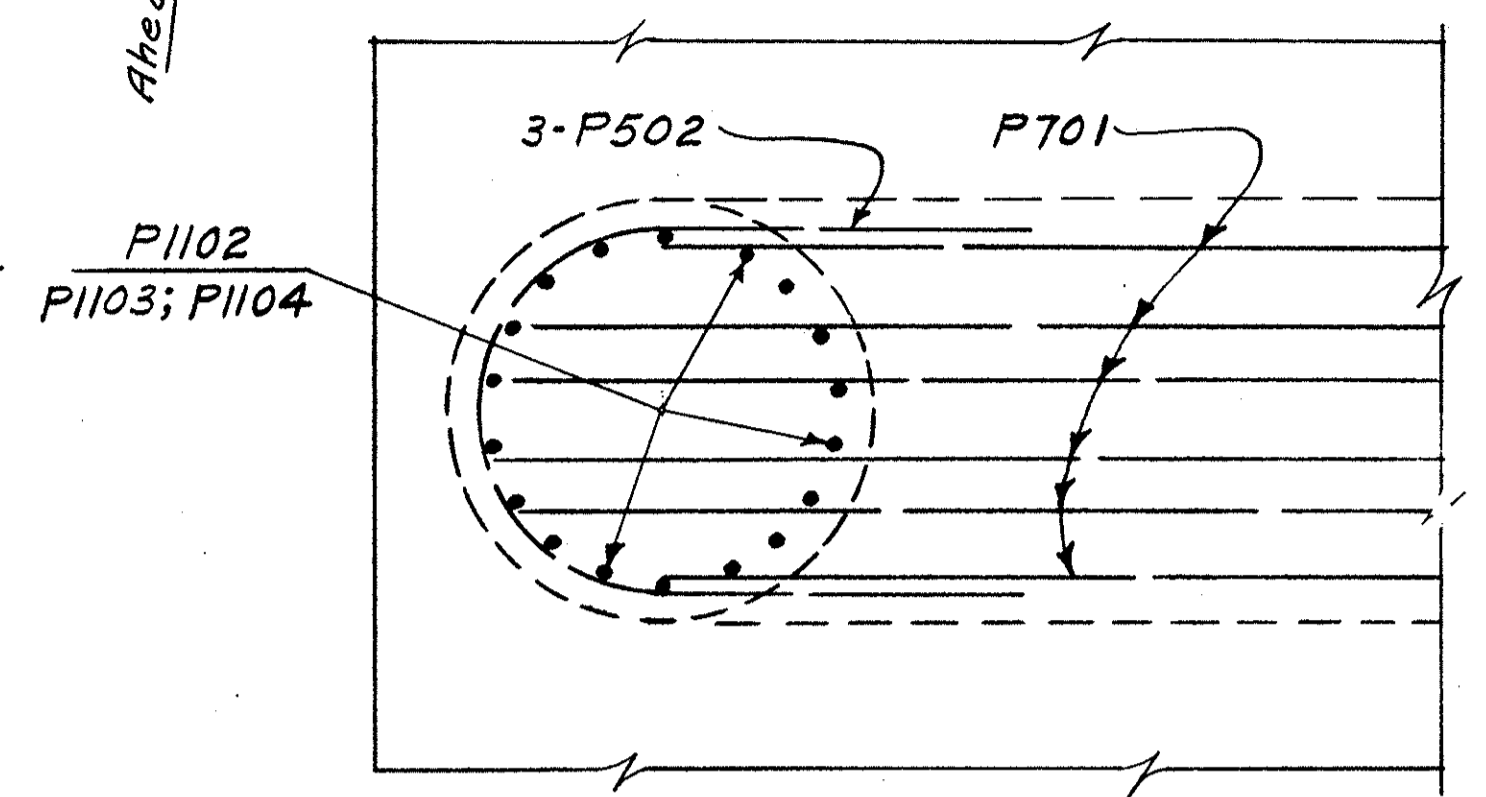
Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
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MAHONING COUNTY
MAH-18-15.50

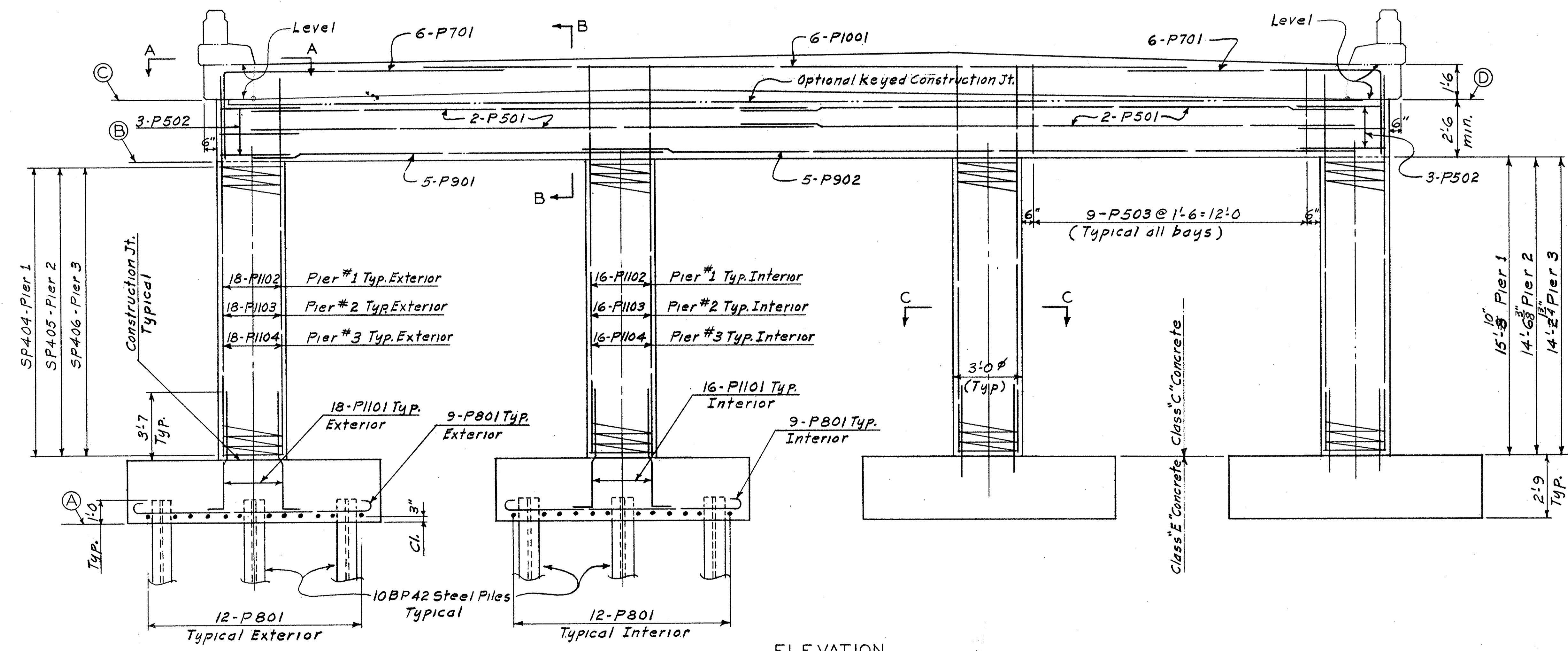


TYPICAL PIER PLAN

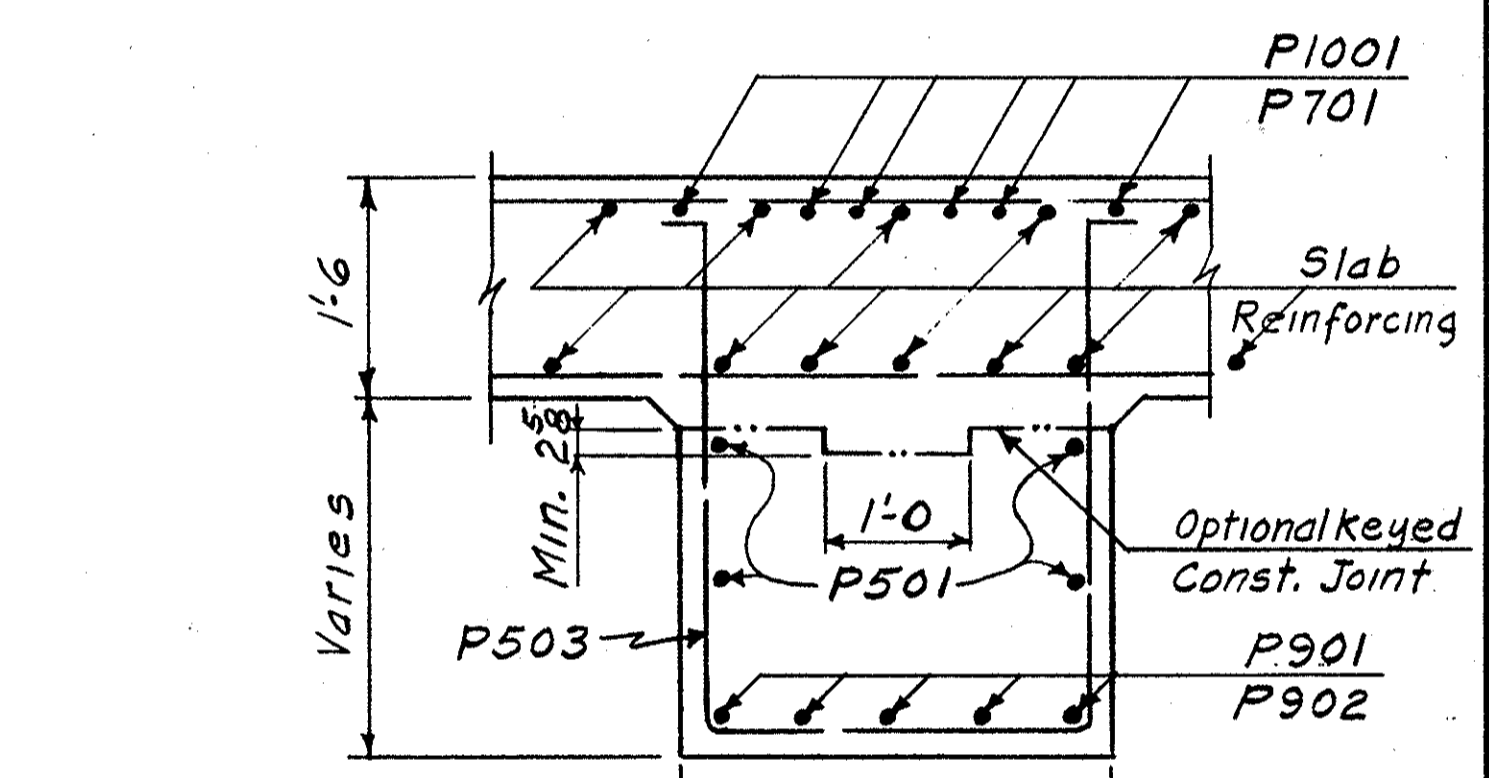


SECTION-AA
Typical

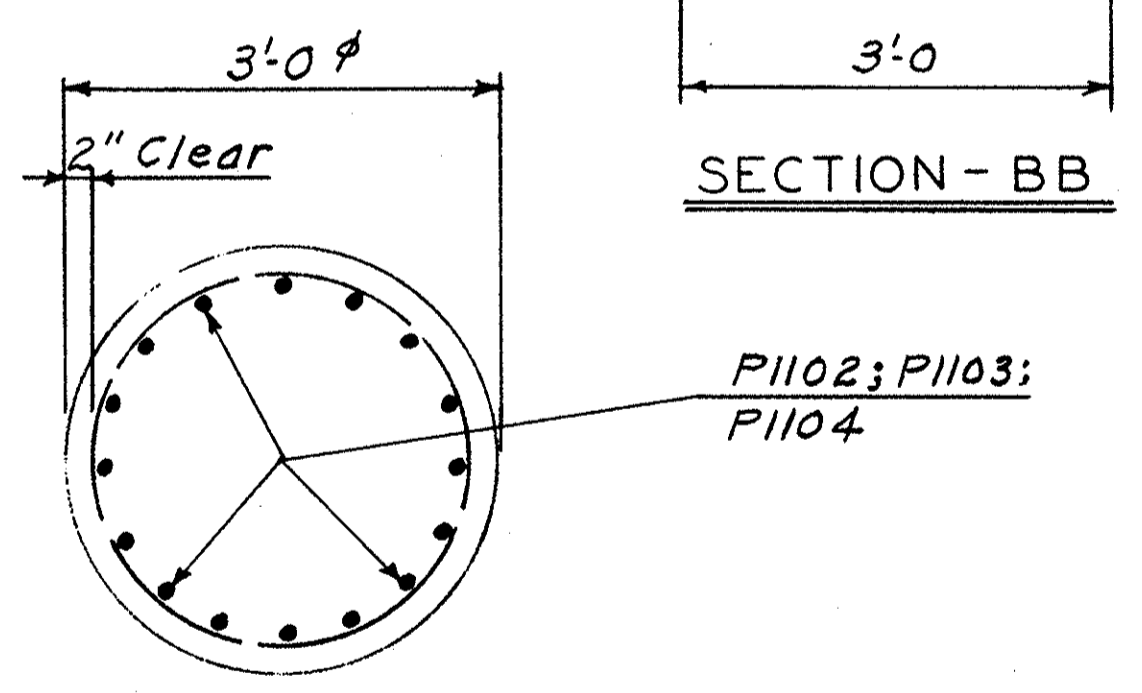
Sta. 519+08.41 Pier # 1
Sta. 519+48.91 Pier # 2
Sta. 519+89.41 Pier # 3



ELEVATION



SECTION-BB



SECTION-CC

See "Electrical Details" sheet for electrical ground wire embedded in columns of Pier # 2 (Sheet 217).

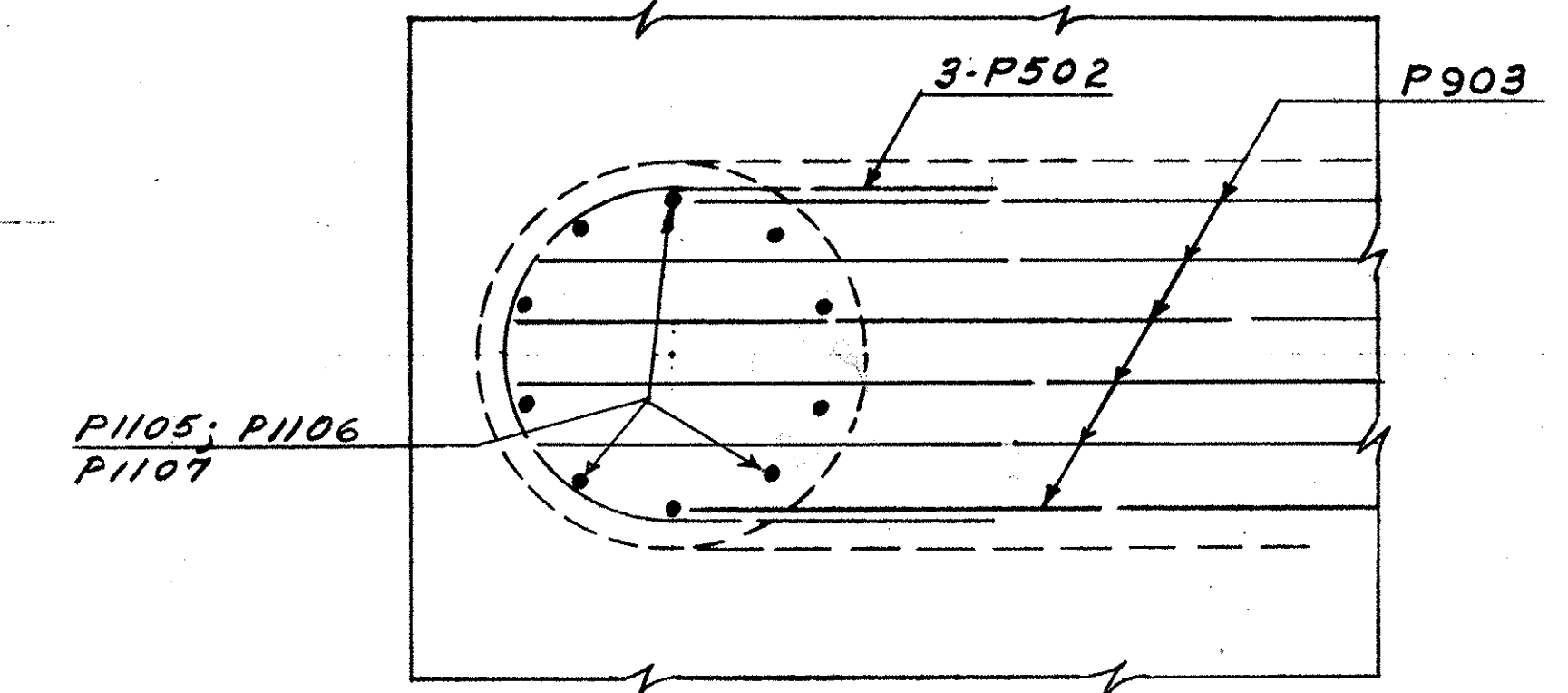
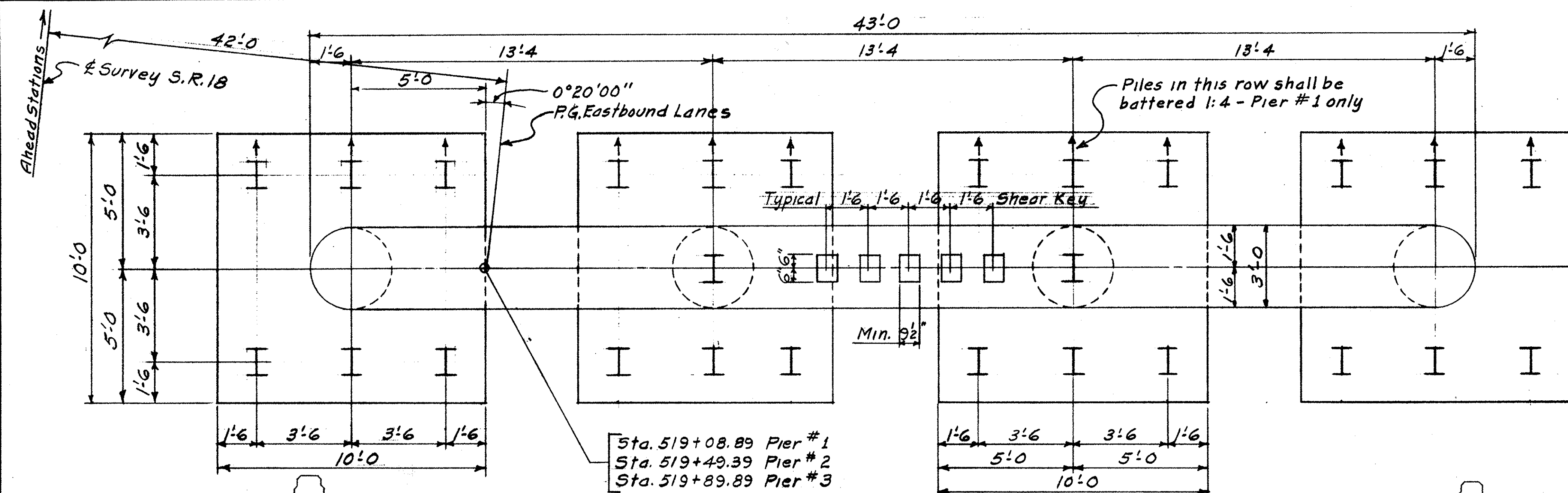
ELEVATIONS							
	A	B	C	D	B	C	D
Pier 1	1000.47	1018.89	1021.39	1021.56	1019.95	1021.63	1021.55
Pier 2	1001.31	1018.56	1021.06	1021.11	1018.57	1021.44	1021.57
Pier 3	1001.18	1018.10	1020.85	1020.60	1018.03	1021.19	1020.53

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ROCHESTER, PENNSYLVANIA

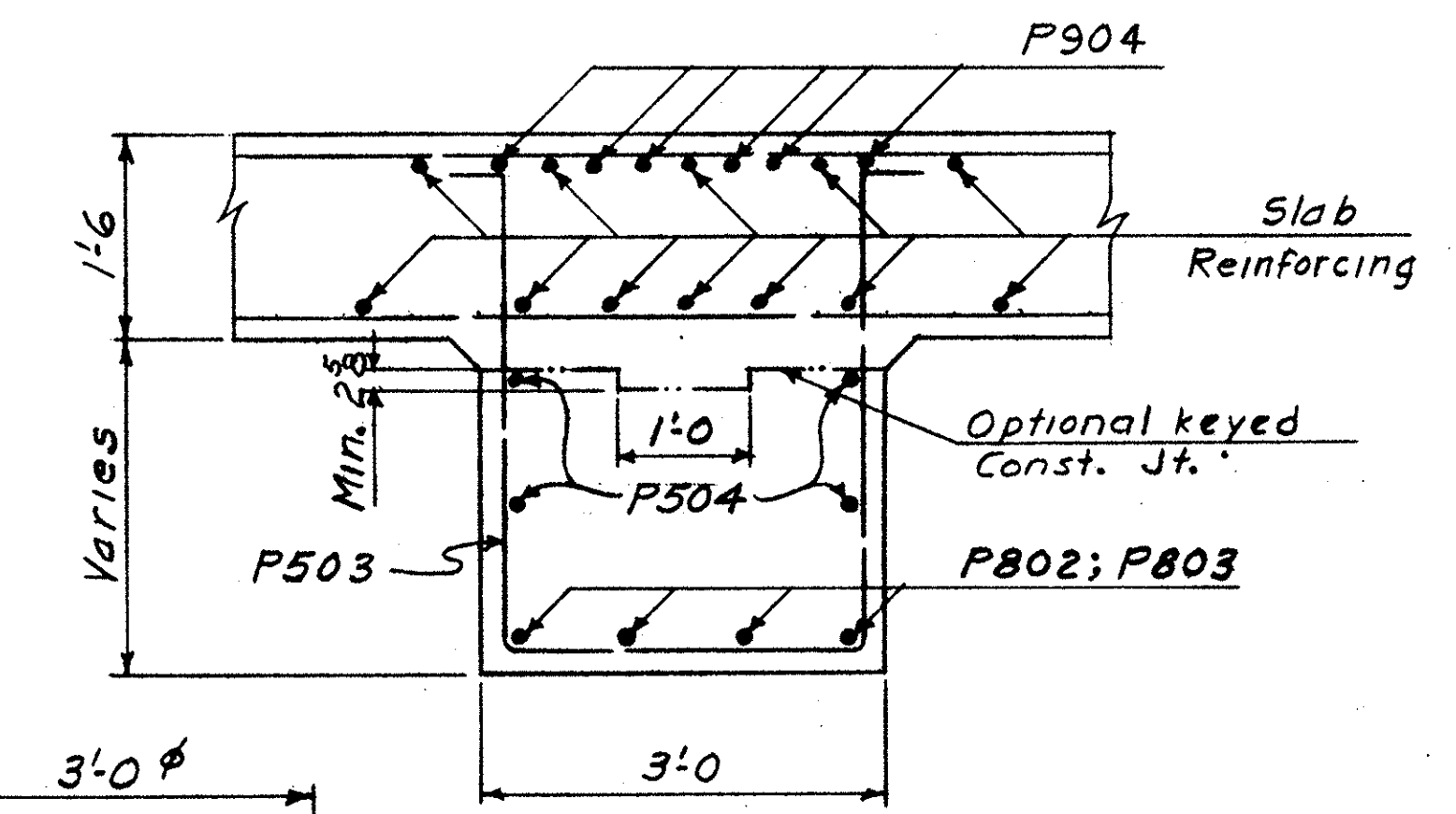
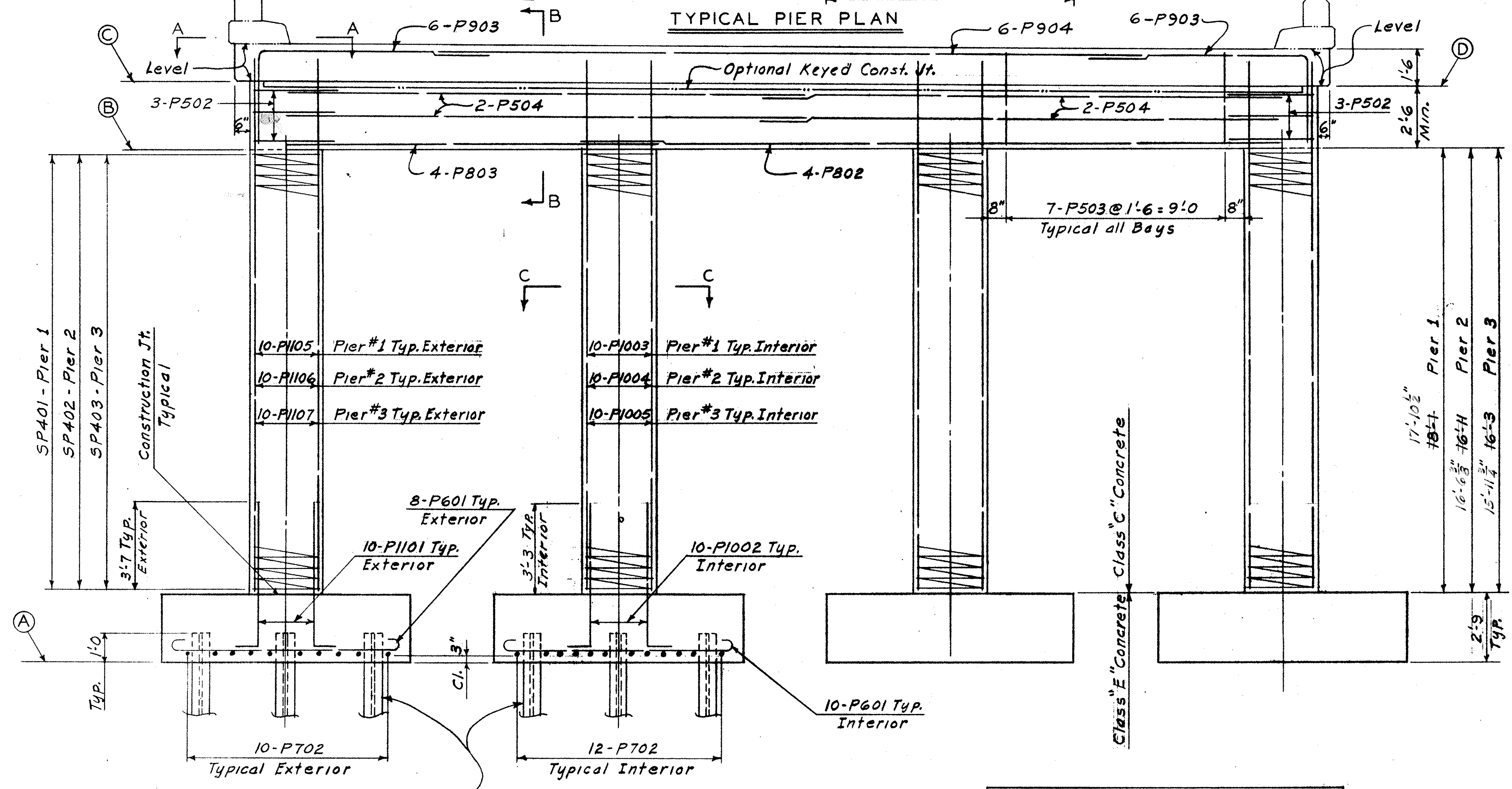
PIER DETAILS
BRIDGE NO. MAH-18-1552L
OVER MERIDIAN ROAD

Sta. 518+75.40 To Sta. 520+22.90

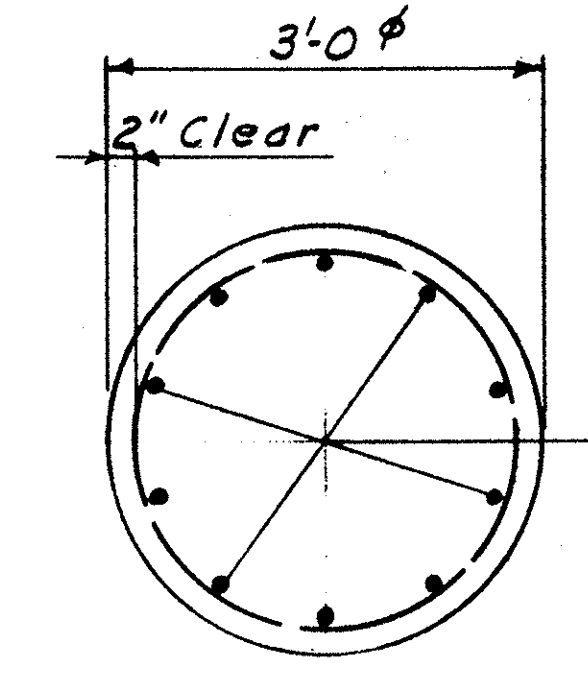
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	TP		AA	L.G.H. 10-15-63	2-1-66



SECTION-AA
Typical



SECTION-BB



SECTION-CC

See "Electrical Details" sheet for electrical ground wire embedded in columns of Pier #2 (Sheet 217).

ELEVATIONS							
	A	B	C	D	B	C	D
Pier 1	998.20	1019.03	1021.58	1021.53	1018.83	1021.59	1021.33
Pier 2	998.99	1018.66	1021.17	1021.16	1018.27	1021.13	1020.77
Pier 3	998.93	1017.93	1020.71	1020.43	1017.66	1020.71	1020.16

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ROCHESTER, PENNSYLVANIA

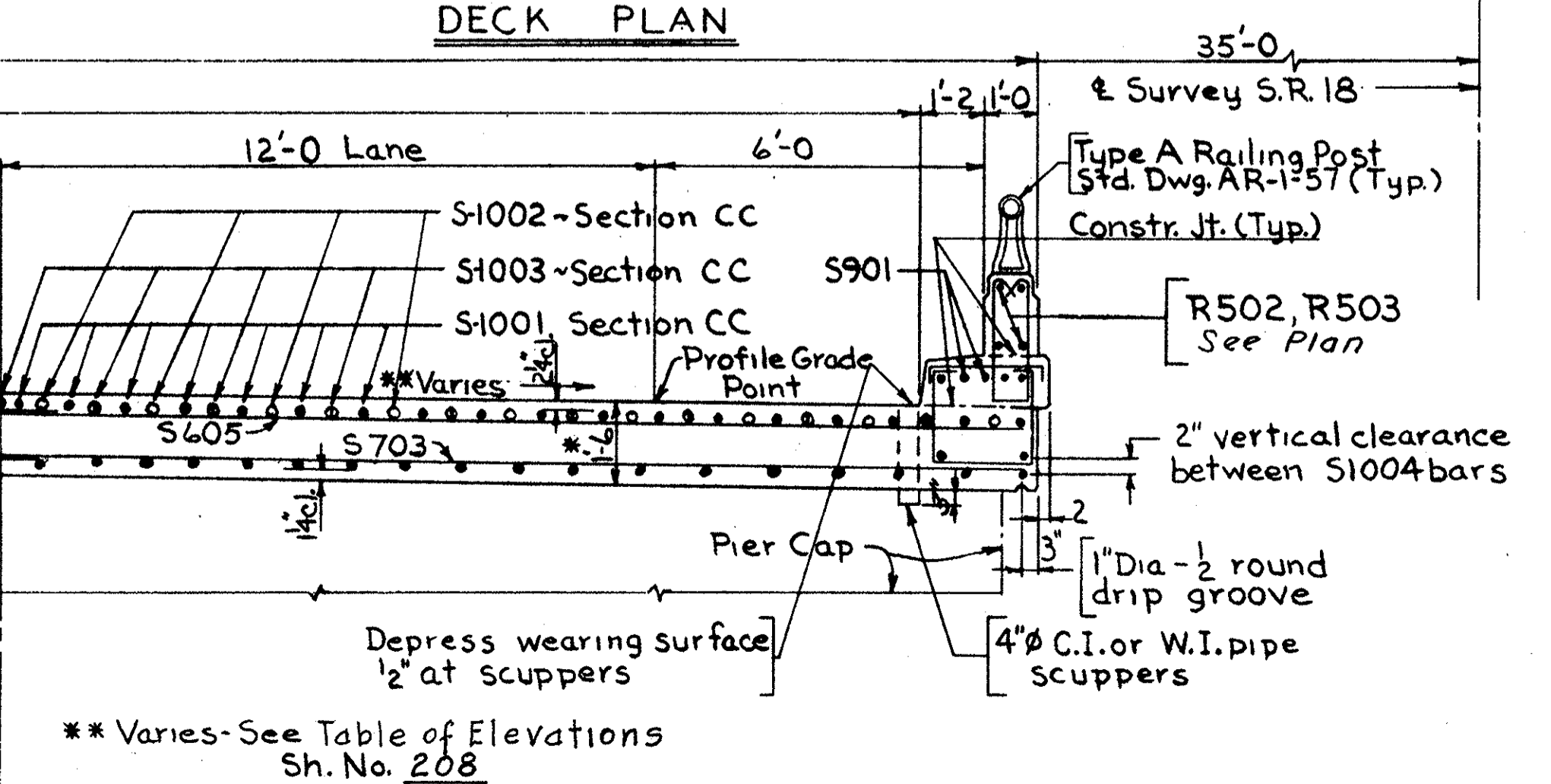
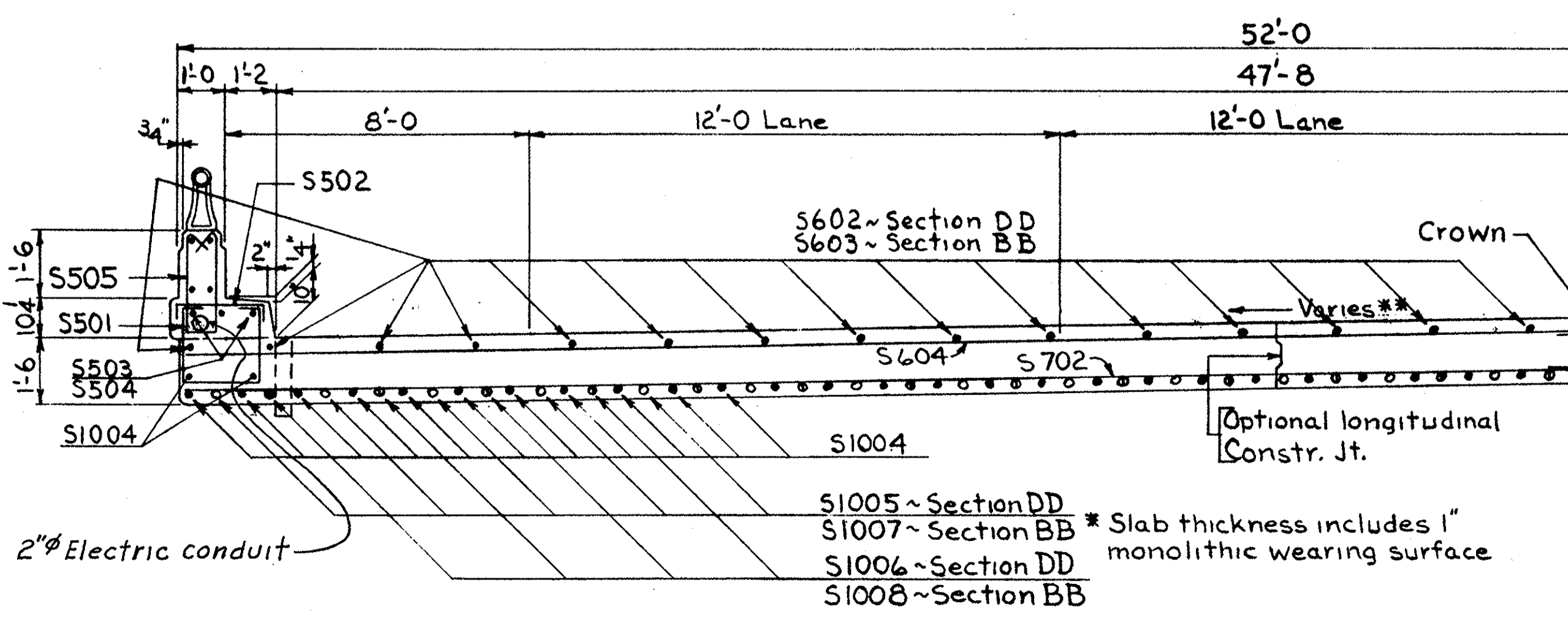
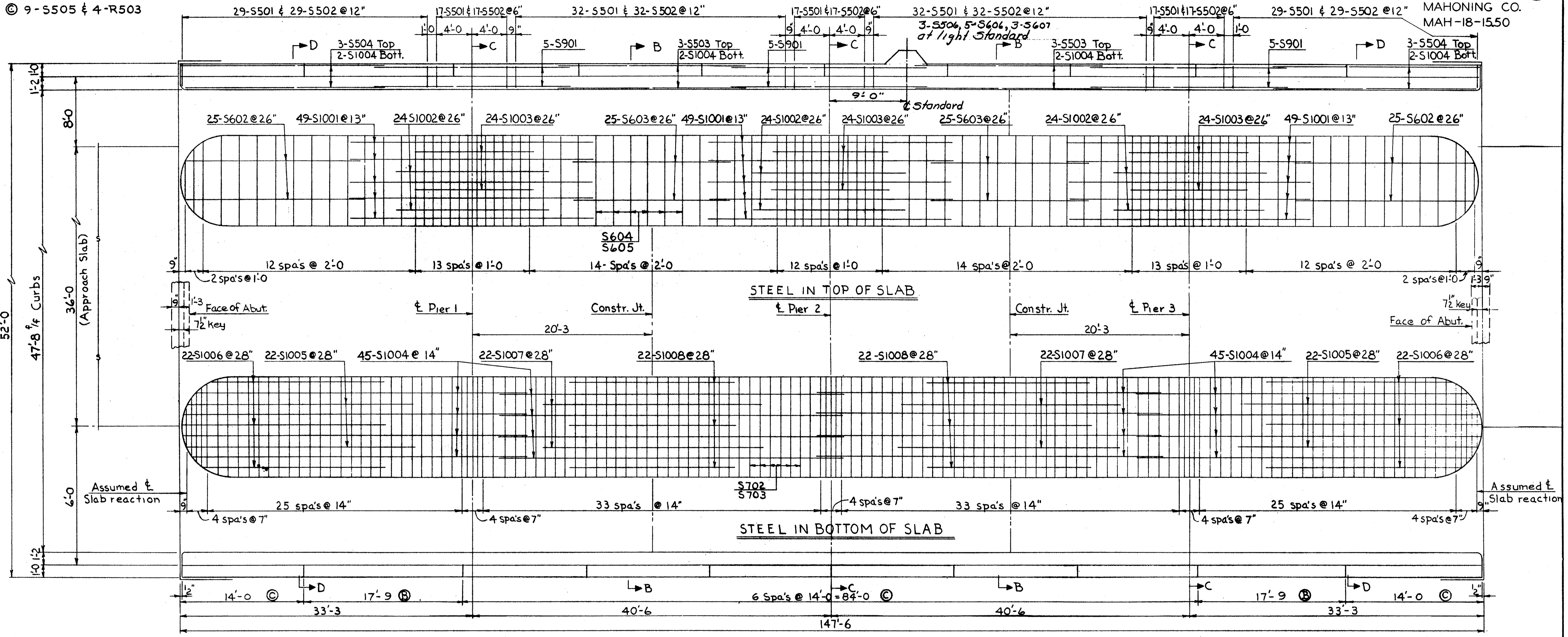
PIER DETAILS
BRIDGE NO. MAH-18-1552R
OVER MERIDIAN ROAD

Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	TP		A.A.	L.G.H. 10-15-63	2-1-66

Ⓟ 12-S505 & 4-R502
 Ⓞ 9-S505 & 4-R503

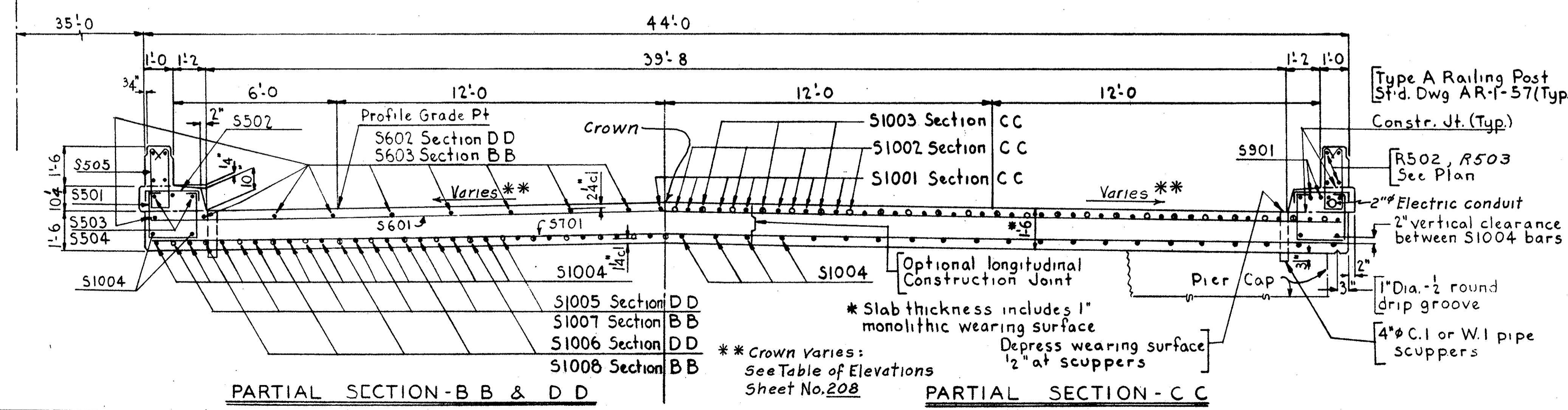
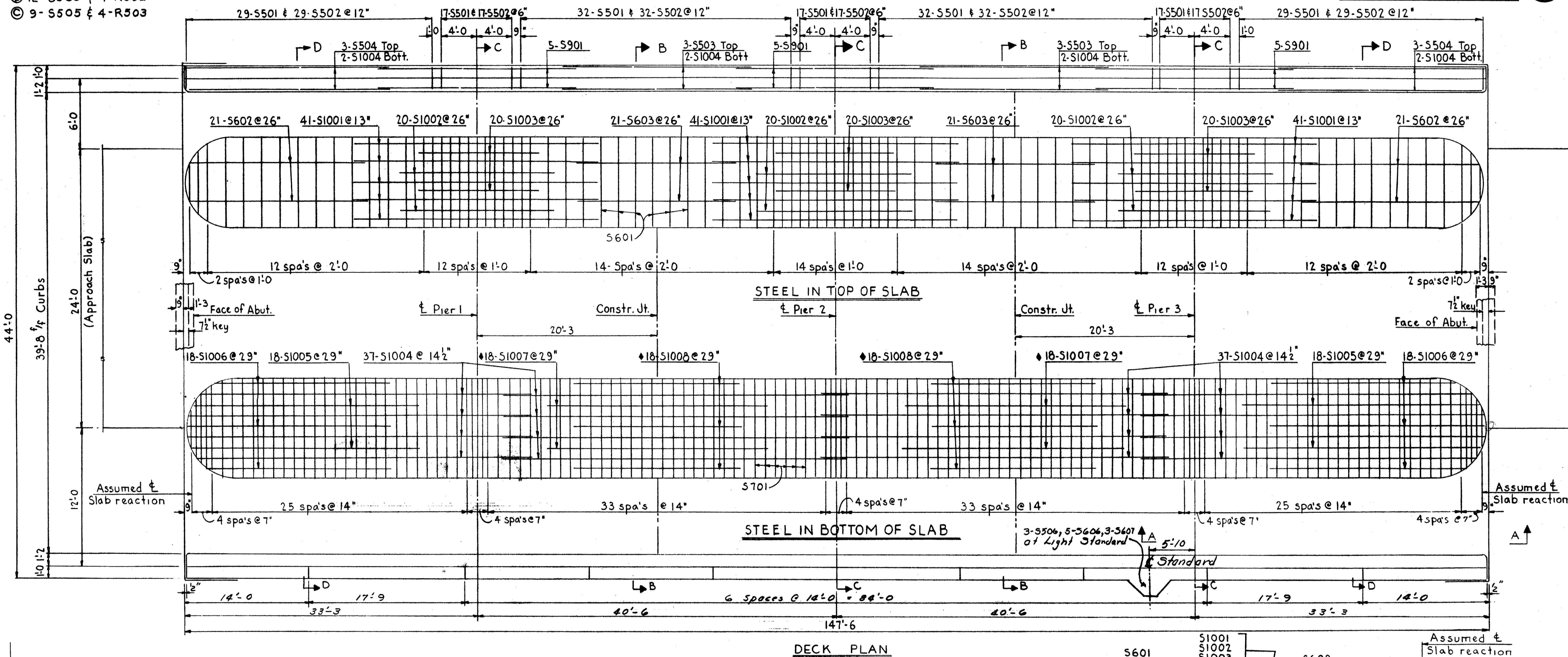
MAHONING CO.
 MAH-18-1550



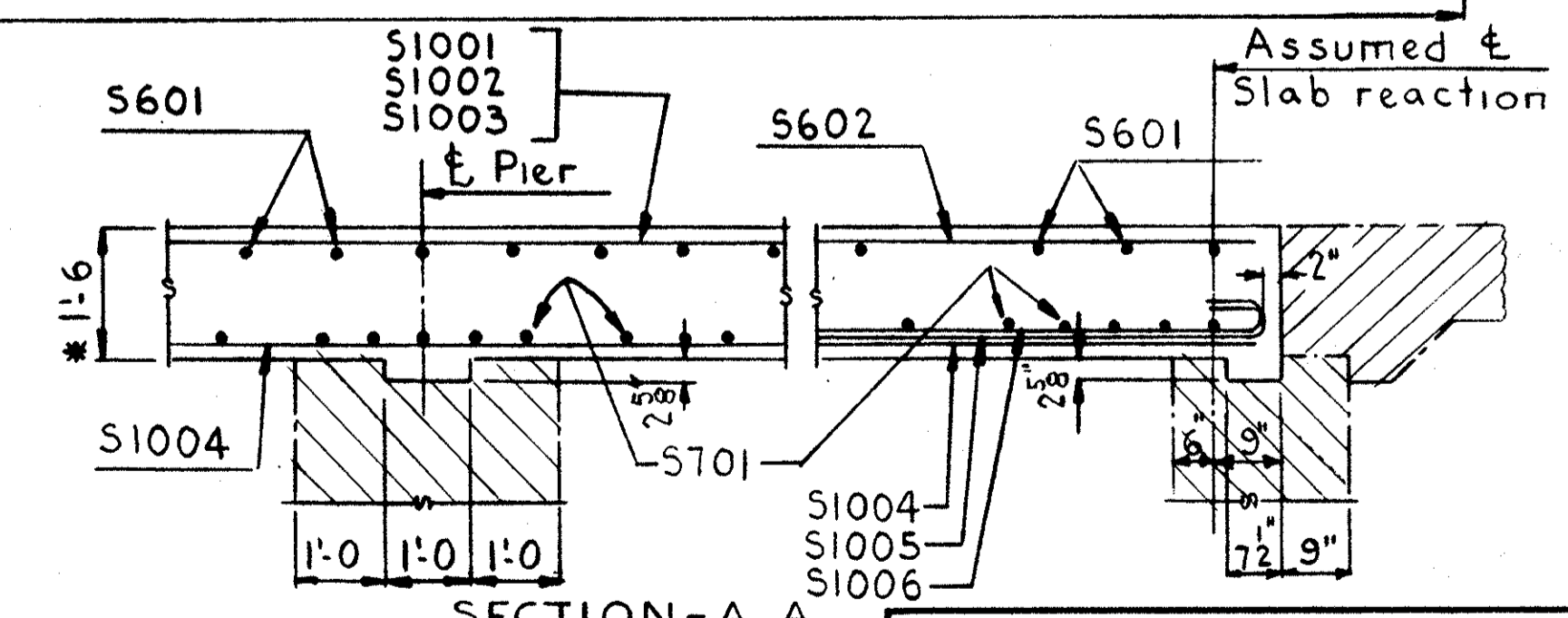
MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
DECK PLAN BRIDGE NO MAH-18-1552L OVER MERIDIAN ROAD					
Sta. 518+75.40 To Sta. 520+22.90					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
C.J.M.	bo		L.P.D. S.J.F.	LGH 10-15-63	

FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

ⓑ 12- S505 & 4-R502
Ⓒ 9- S505 & 4-R503



- NOTES:**
- Curbs shall be placed after the shoring under the slab has been released sufficiently to permit the slab spans to attain full dead load deflection
 - Camber: Loaded falsework shall have a camber of 1/2" for end spans and 3/4" for the middle spans to allow for vertical curve and dead load deflection. This is the amount of camber required before falsework is released. To obtain this proper allowance shall be made for the deflection of falsework members.
 - Concrete shall be class 'C'



MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

DECK PLAN
BRIDGE NO MAH-18-1552R
OVER MERIDIAN ROAD

Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
C.J.M.	to		LRD S.J.F.	2/6/63 10-15-63	

MAHONING COUNTY
MAH-18-15.50

REINFORCING STEEL LIST

NOTES
Bar size is indicated in the bar mark. The first digit, where three digits are used, indicate the bar size number. For example S701 is a No. 7 in size.

Spiral Reinforcing Bars: The "length" shown in the steel list for the spiral bars is the distance from the top of the footing to the bottom of the pier cap. The "No. of Turns" shown is the "length" divided by the pitch, plus three turns (total number of closed coils), expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item S-4. 1 1/2 closed coils shall be provided at the ends of each spiral unit. Four steel channel, tee or angle steel spacers, weighing approximately 0.68 lbs. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantity of spiral bars.

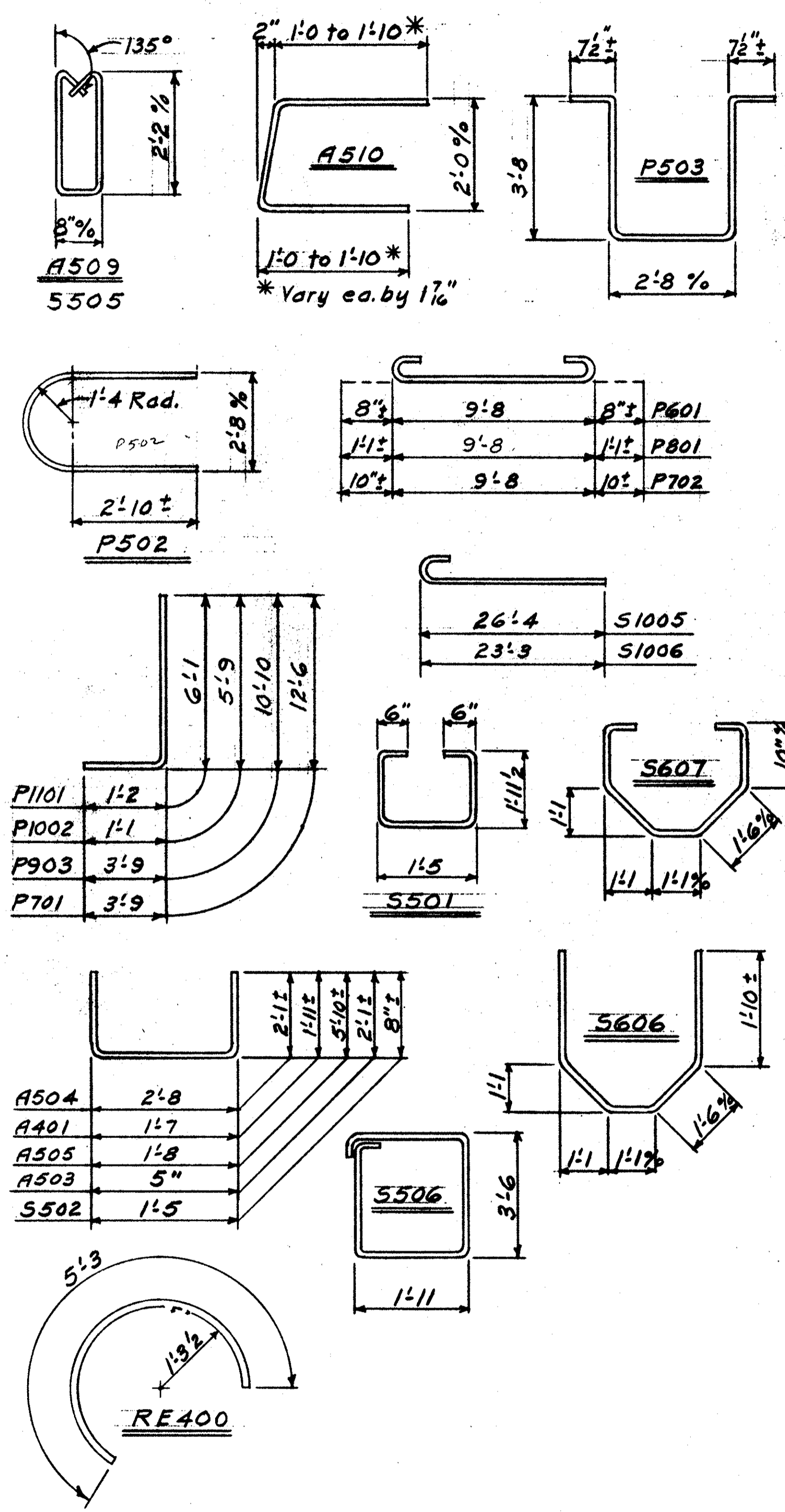
ELECTRICAL NOTES:
Electrical Ground: Use No. 4/0 AWG 7 strand, soft annealed bare copper cable encased in the outside column of Pier No. 3 right and Pier No. 2 left. The lower end of the cable shall be exothermic welded to one of the steel piles in the footing and the upper end shall extend far enough so that it may be exothermic welded to a light pole anchor rod.

For bridge lighting details, see Details - Bridge Lighting, Sheet No. 203.

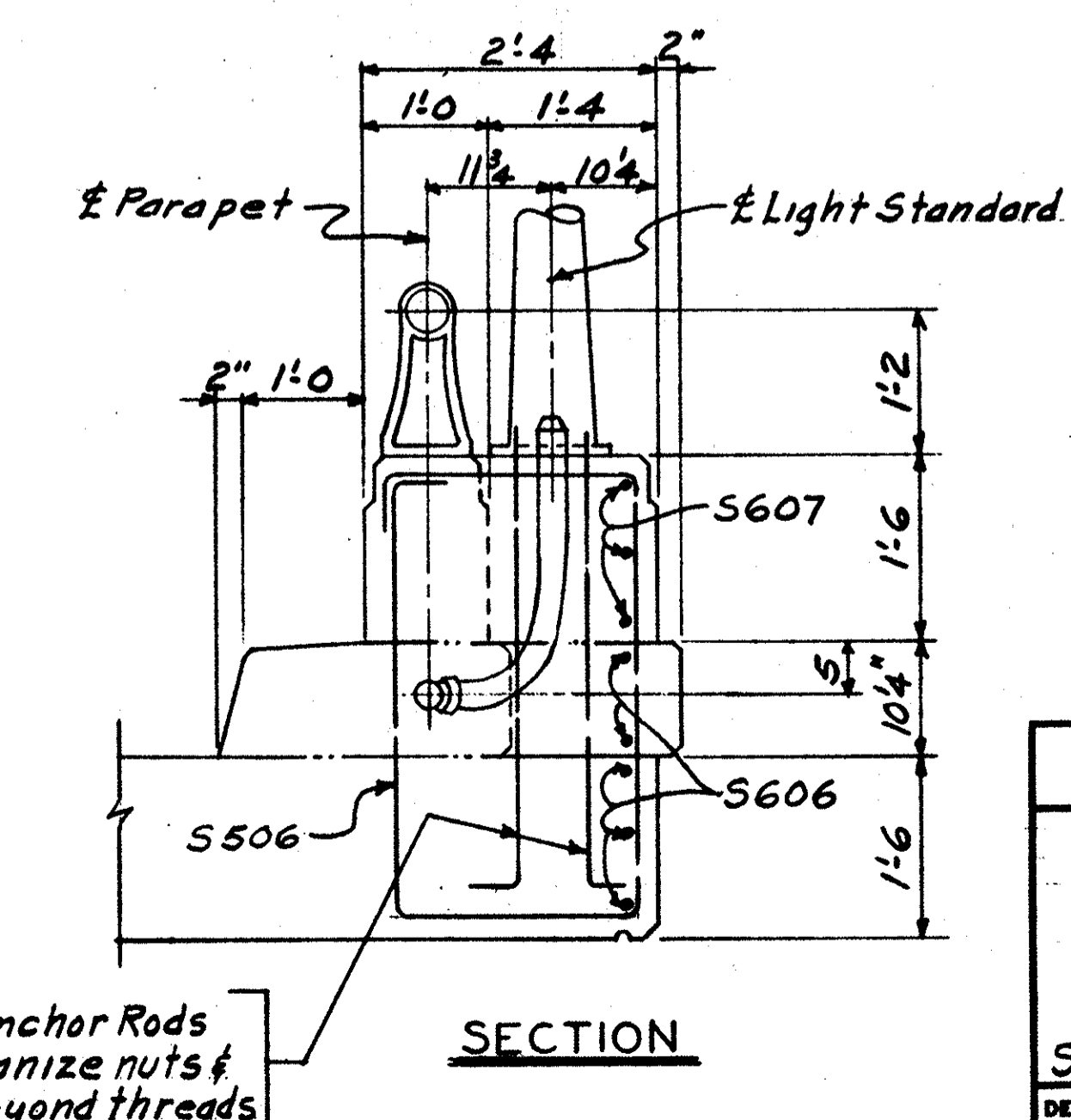
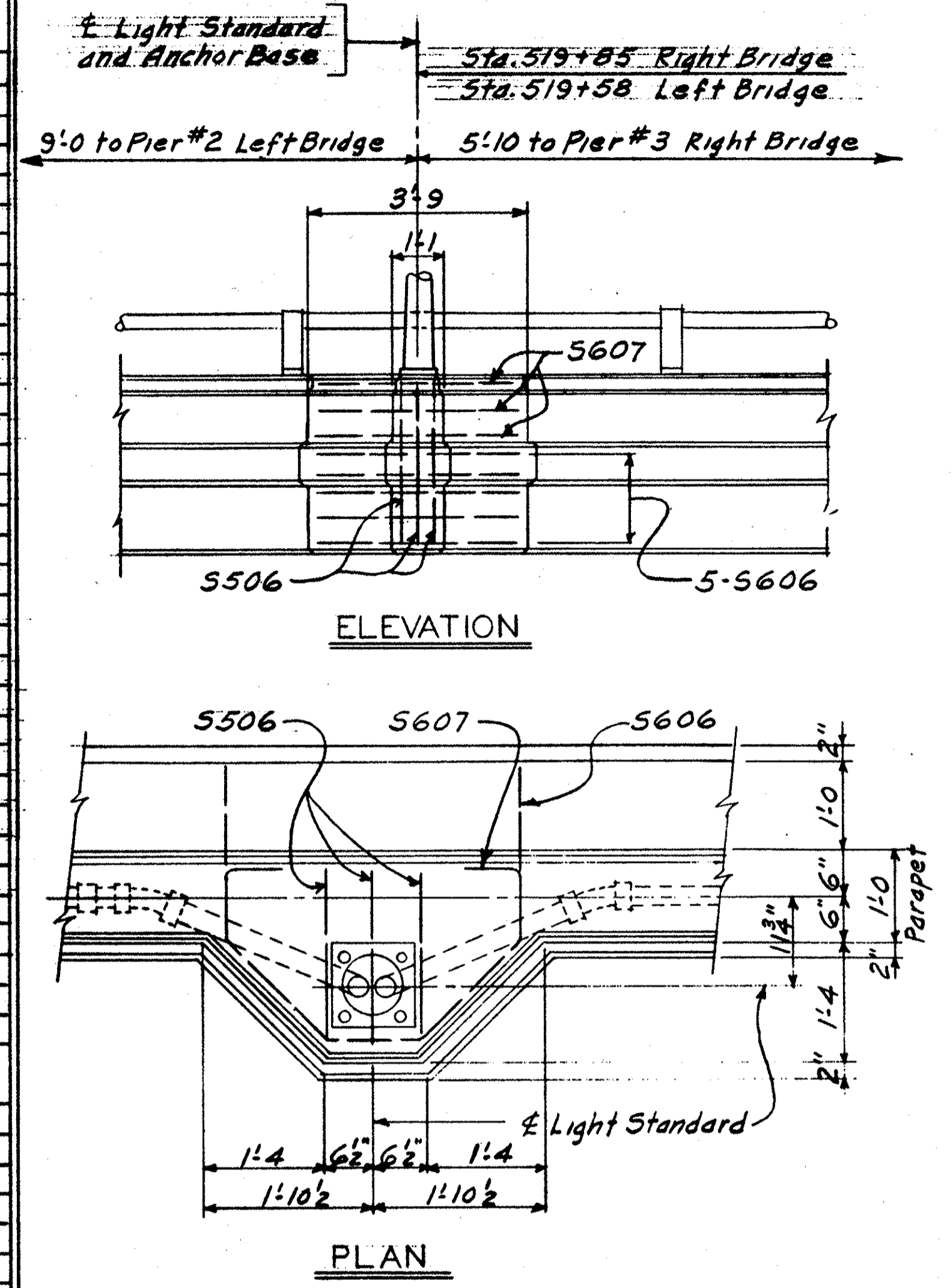
MARK	No. Lt.	No. Rt.	Length	Weight Lt.	Weight Rt.	Sh'p.
SUPERSTRUCTURE						
S1001	147	123	27'11"	17659	14777	S
S1002	72	60	12'11"	4002	3335	S
S1003	72	60	9'4"	2892	2409	S
S1004	196	164	37'8"	31768	26581	S
S1005	44	36	27'9"	5254	4298	B
S1006	44	36	24'8"	4670	3821	B
S1007	44	36	24'10"	4702	3847	S
S1008	44	36	19'2"	3629	2969	S
S901	30	30	22'6"	2295	2295	S
S701		137	43'8"		12227	S
S702	197		34'0"	9521		S
S703	197		20'0"	5600		S
S601		95	43'8"		6231	S
S602	50	42	20'11"	1571	1320	S
S603	50	42	17'8"	1327	1115	S
S604	95		33'10"	4828		S
S605	95		19'10"	2830		S
S606	5	5	7'6"	56	56	B
S607	3	3	6'6"	29	29	B
S501	346	346	5'10"	2105	2105	B
S502	346	346	2'6"	902	902	B
S503	12	12	21'8"	271	271	S
S504	12	12	23'8"	296	296	S
S505	192	192	5'7"	1118	1118	B
S506	3	3	11'6"	36	36	B
R501	16	16	9'8"	**	**	S
R502	16	16	17'5"	**	**	S
R503	64	64	13'8"	**	**	S
ABUTMENTS						
A1001		16	23'3"		1600	S
A1002	16		27'4"	1882		S
A801		16	23'8"		1012	S
A802	16	16	7'10"	334	334	S
A803	16		27'8"	1182		S
A501		8	12'6"		104	S
A502	8	8	6'6"	54	54	S
A503	22	28	4'4"	100	126	B
A504	216	176	6'7"	1484	1208	B
A505	70	60	13'1"	956	818	B
A506		16	22'5"		374	S
A507		16	23'2"		386	S
A508	16	16	7'10"	130	130	S
A509	32	32	5'7"	186	186	B
A510	45 of 8	45 of 8	3'9" to 3'5"	152	152	B
A511	32	32	9'10"	328	328	S
A512	40	40	3'0"	126	126	S
A513	48	48	7'9"	388	388	S
A514	16	16	6'4"	106	106	S
A515	16	16	11'2"	186	186	S
A516	8		8'6"	70		S
A517	16		26'6"	442		S
A518	16		27'3"	454		S
A401	88	72	5'3"	308	252	B

Mark	No. Lt.	No. Rt.	Core Dia.	Length	No. Turns	Pitch	Weight Left	Weight Right
SP401	4		32"	18'1"	51	4 1/2"		1321
SP402		4	32"	16'11"	48		1242	
SP403	4		32"	16'3"	46		1190	
SP404	4		32"	15'8"	45		1162	
SP405	4		32"	14'6"	42		1083	
SP406	4		32"	14'2"	41	4 1/2"	1058	

** Included with railing for payment (Item S-14)



MARK	No. Lt.	No. Rt.	Length	Weight Lt.	Weight Rt.	Sh'p.
PIERS						
P1101	204	60	6'11"	7497	2205	B
P1102	68		19'3"	6955		S
P1103	68		18'1"	6533		S
P1104	68		17'9"	6413		S
P1105	20		21'8"		2302	S
P1106	20		20'6"		2178	S
P1107	20		19'10"		2107	S
P1001	18		32'0"	2479		S
P1002	60		6'6"		1678	B
P1003	20		21'4"		1836	S
P1004	20		20'2"		1736	S
P1005	20		19'6"		1678	S
P901	15		17'5"	888		S
P902	15		33'5"	1704		S
P903	36		14'4"		1754	B
P904	18		26'8"		1632	S
P801	252		11'10"	7962		B
P802	12		27'11"		894	S
P803	12		14'7"		467	S
P701	36		16'1"	1183		B
P702	132		11'4"		3058	B
P601	108		11'0"		1784	B
P501	24		24'10"	622		S
P502	18	18	9'10"	185	185	B
P503	81	63	10'9"	908	706	B
P504	24		20'10"		522	S
REPLACEMENT BARS						
RE1100	2	1	7'7"	81	40	S
RE1000	4	4	7'3"	125	125	S
RE900	1	1	6'10"	23	23	S
RE800	1	1	6'6"	17	17	S
RE700	1	1	6'3"	13	13	S
RE600	1	1	5'11"	9	9	S
RE500	1	1	5'7"	6	6	S
RE400	1	1	5'3"	4	4	B



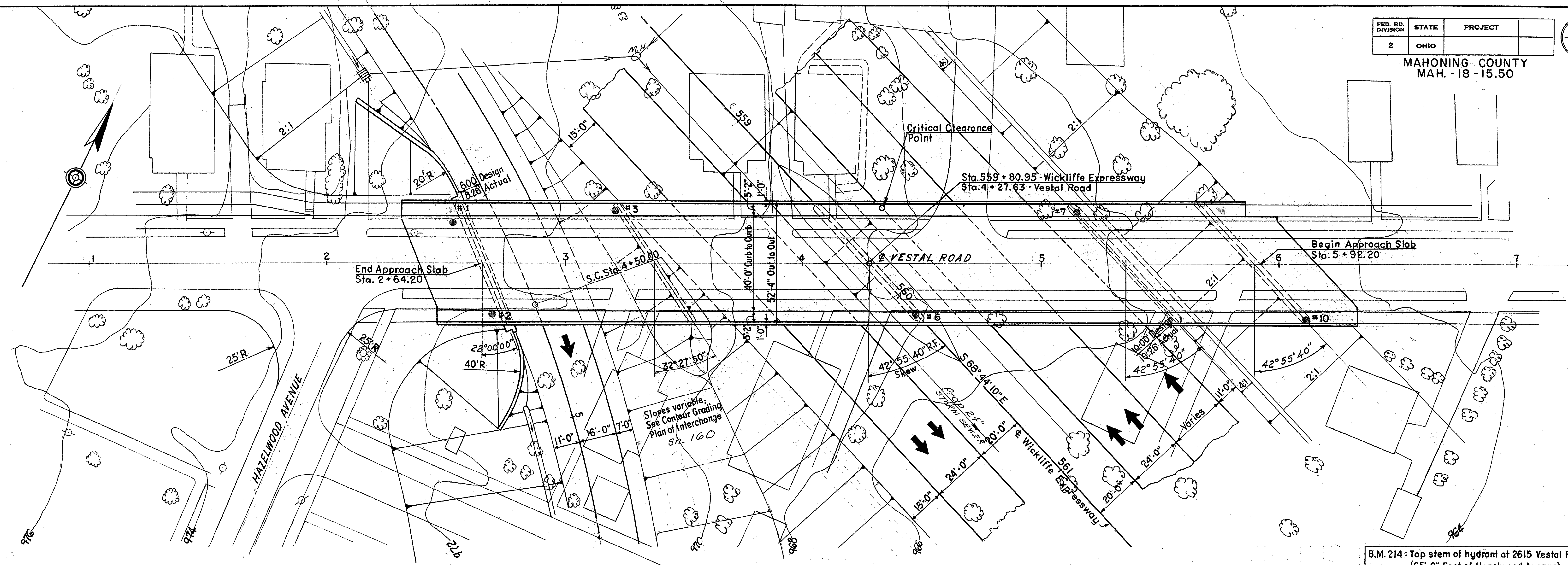
2-Light Standard Anchor Rods (1" or 1 1/4" x 40") Galvanize nuts & exposed ends 2" beyond threads

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

REINFORCING STEEL LIST
& ELECTRICAL DETAILS
BRIDGE NO. MAH-18-1552L&R
OVER MERIDIAN ROAD
Sta. 518+75.40 To Sta. 520+22.90

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	TP		SJF	LGH 10-15-63	

MAHONING COUNTY
MAH. - 18 - 15.50



FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office but the State assumes no responsibility for the accuracy thereof.

See Sh. 159 for temporary run-around

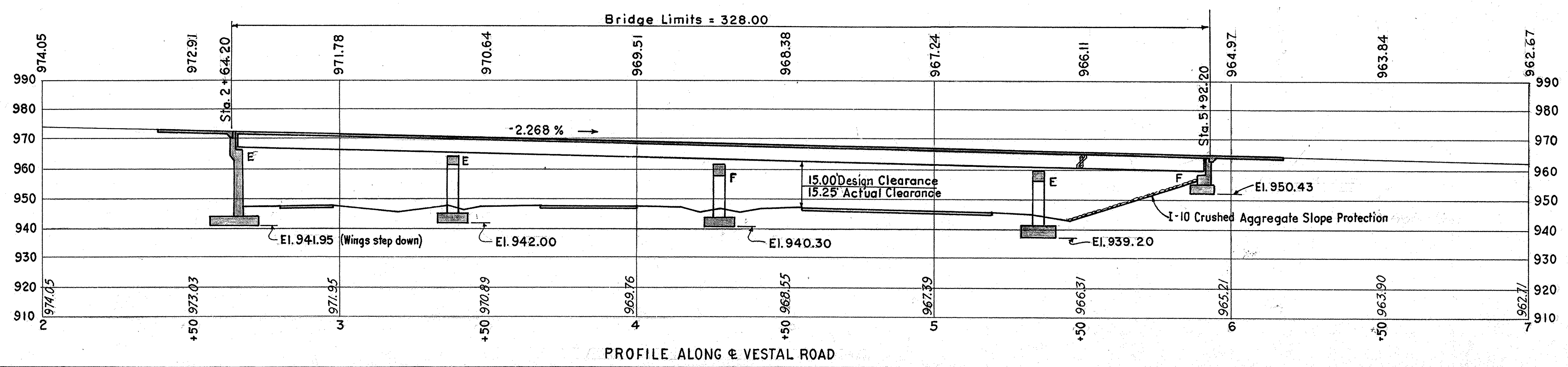
B.M. 214: Top stem of hydrant at 2615 Vestal Road
(65'-0" East of Hazelwood Avenue)
Elevation: 975.526

B.M. 209: Bolt on top of hydrant at 2434 Vestal Rd.
(135'-0" East of Hazelwood Avenue)
Elevation: 958.902

ADT 10,090 (1975)

PROPOSED STRUCTURE

TYPE: Continuous Steel Girders with reinforced concrete deck and superstructure.
SPANS: 71'-0"; 90'-0"; 107'-6" & 54'-0" (along E Vestal Rd.)
ROADWAY: 40' 1/4" 5'-2" Sidewalks
LOAD FREQUENCY: C.F. 400 (57)
SKEW: 42°55'40" R.F. except as shown
WEARING SURFACE: Monolithic Concrete
APPROACH SLAB: A.S.-1-54 (25' Long)
ALIGNMENT: Tangent



MICHAEL BAKER, JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SITE PLAN
BRIDGE NO. MAH.-18-1629
UNDER VESTAL ROAD
MAHONING COUNTY
STATION 559 + 80.95

PRESENT TOPOGRAPHY		PROPOSED WORK		
SURVEYED	DRAWN	DESIGNED	DRAWN	CHECKED
Avillio	M.A.M.	JFK	JFK	E.F.R.
				L.G.H.

MAHONING COUNTY
MAH-18-15.50

GENERAL NOTES:
 Design Loading - CF 400 (57)
 Concrete Class C - basic unit stress 1,333 p.s.i.
 Concrete Class E - basic unit stress 1,333 p.s.i.
 Structural Steel - ASTM A36 - basic unit stress 20,000 p.s.i. (ASTM A7 and A373 steel not permitted).
 Reinforcing Steel - ASTM A15, A16, A160, deformed, intermediate or hard grade, basic unit stress 20,000 p.s.i. except spiral reinforcement may be plain, structural grade with basic unit stress of 18,000 p.s.i.

REFERENCE shall be made to Standard Drawings AS-1-54, revised 7-5-62; CSB-2-56, sheets 2 and 3 of 6, revised 2-2-59; AR-1-57, revised 4-2-62; FSB-1-62, revised 1-15-63 and Supplemental Specifications 5-101 dated 7-12-62, and S-307 revised 10-1-64.

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of OHIO, Department of Highways, Dated 9-1-57, together with current revisions thereof.

FOUNDATION BEARING PRESSURE: Rear abutment and pier footings are designed for a maximum bearing pressure of 2.9 tons per square foot. Wing walls C and D are designed for a maximum bearing pressure of 2.5 tons per square foot. Forward abutment footing is designed for a maximum bearing pressure of 2.2 tons per square foot.

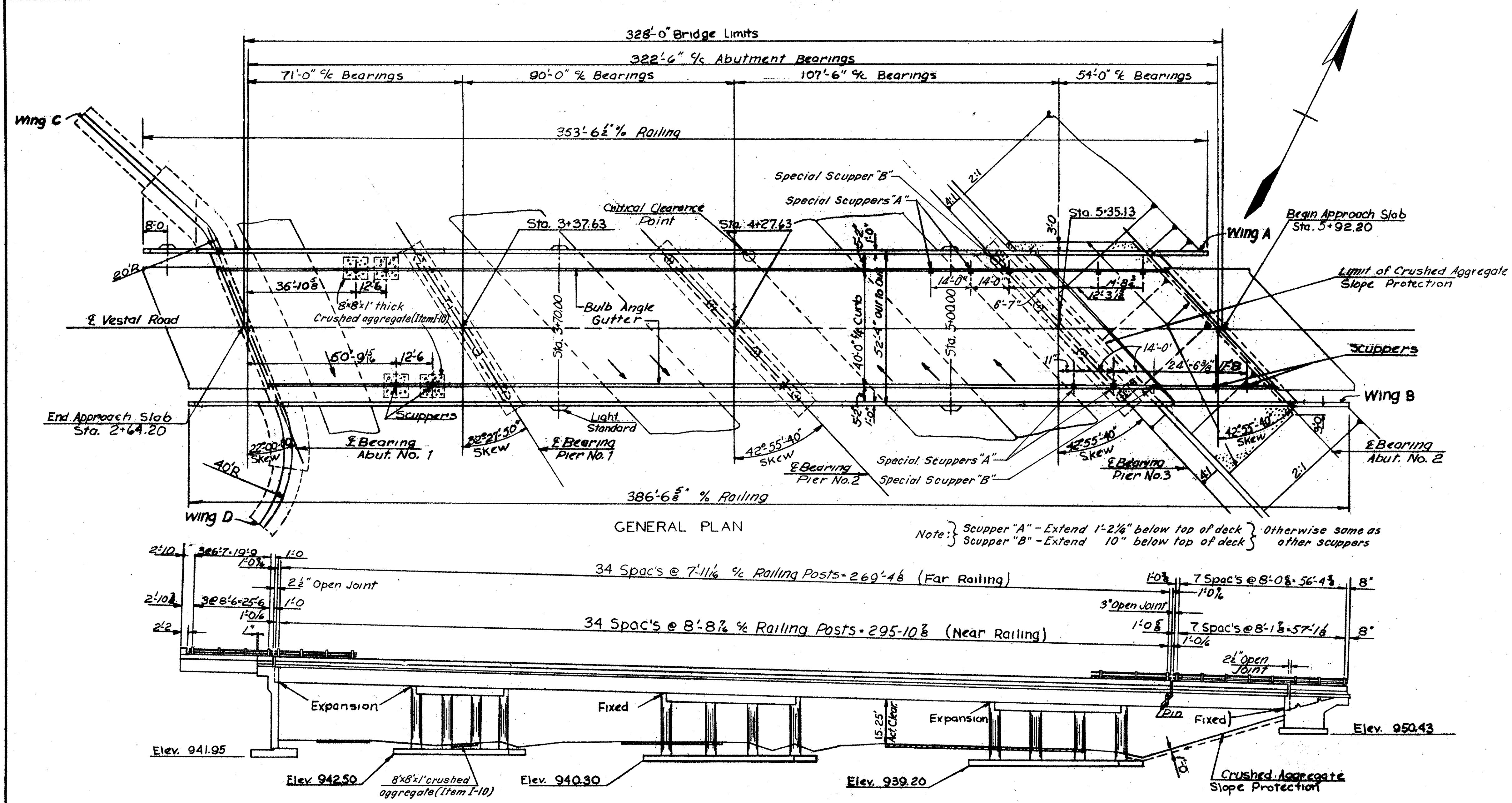
WELDING of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.

CONCRETE DECK PLACING: In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress up grade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.

SURFACE FINISH OF CONCRETE: The requirements of Section S-1.22, Rubbed Finish, shall apply to the following concrete surfaces:
 (a) The entire superstructure except top and bottom surfaces of sidewalks and roadways.
 (b) The entire exposed surface of piers, abutments and wing walls except bearing seats, backwalls, top of rear abutment wing walls and face of forward abutment between outside girders.

MACHINE FINISH: The concrete deck shall be finished by the use of a finishing machine.

S-7.10 High STRENGTH STEEL BOLTS: High strength bolts that are used as fitting up bolts may be used in the final assembly if, after all bolts are placed and tightened, the fitting up bolts are subsequently loosened and retightened by the turn of nut method.



Note: Scupper "A" - Extend 1'-2 1/2" below top of deck } Otherwise same as
 Scupper "B" - Extend 10" below top of deck } other scuppers

ESTIMATED QUANTITIES

Item	Total	Unit	Description	Superstr.	Abut. 1	Abut. 2	Piers	Wings/D	General
E-2	Lump sum	Lump sum	Cofferdams, cribs and sheeting						lump sum
E-2	2173	cu. yd.	Unclassified excavation		793	242	386	752	
S-1	581	cu. yd.	Class "C" concrete, superstructure	581					
S-1	145	cu. yd.	Class "C" concrete, pier caps & columns				145		
S-1	372	cu. yd.	Class "E" concrete, footings		97		159	116	
S-7	151	cu. yd.	Class "E" concrete, abutment #2			151			
S-1	313	cu. yd.	Class "E" concrete, abutment #1 & Wings		166			147	
S-3	137	lin. ft.	Waterproofing, Premould Sealing Strip		125	12			
S-4	270,473	lbs.	Reinforcing steel	147091	35724	7015	63,006	17637	
S-7	588,800	lbs.	Structural steel	588,800					
S-8	588,800	lbs.	Field painting of structural steel	588,800					
S-9	152	sq. ft.	1" preformed expansion joint filler		152				
S-14	739.18	lin. ft.	Railing Type C (Aluminum railing and supports and concrete parapet and end posts)	648.98	52.95	37.25			
S-25			*Electrical - Lighting System						
S-29	249	cu. yd.	Porous backfill		111	33		105	
S-29	14	each	Scuppers (8 standard plus 6 Special)	14					
S-29	36	lin. ft.	8" bituminous coated corrugated metal pipe		36				
S-29	58.5	lin. ft.	6" standard pipe horizontal conductors, Wrought Iron or hot dip galvanized steel including specials	58.5					
S-29	42.5	lin. ft.	6" standard pipe down spout, Wrought Iron or hot dip Galv. Steel, including specials	42.5					
S-101	581	each	Water-reducing, set-retarding admixture	581					
I-10	368	sq. yd.	Crushed aggregate slope protection						368

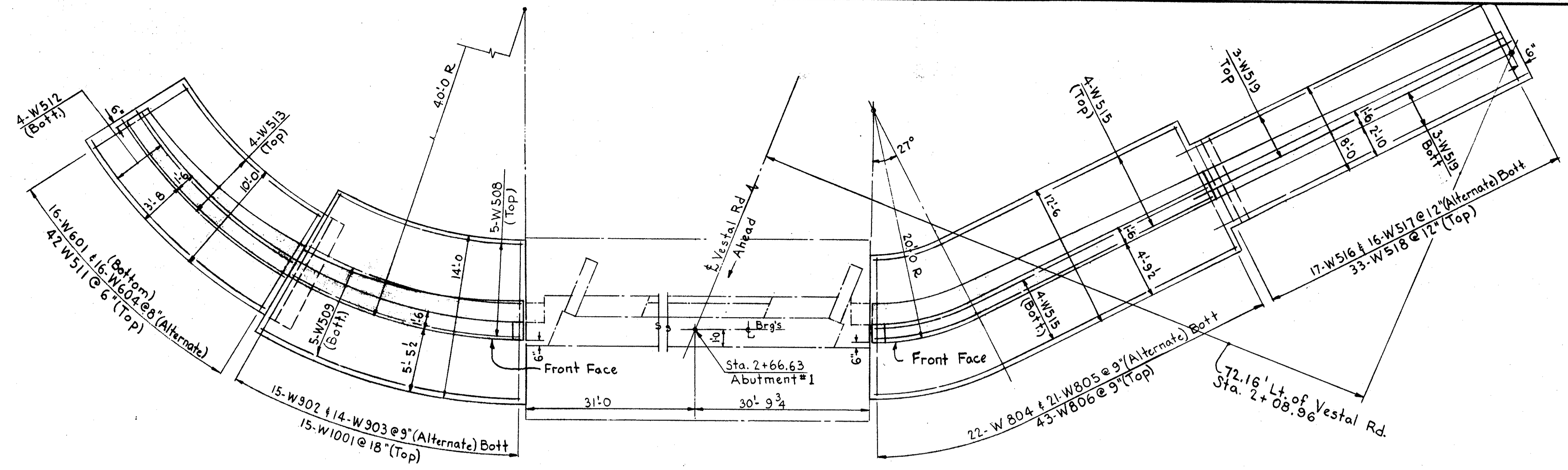
* See Sheets 195 to 206 for Item S-25 Specifications, Details & Quantities.

MICHAEL BAKER JR., CONSULTING ENGINEERS
 ROCHESTER, PENNSYLVANIA

GENERAL PLAN, ELEVATION, NOTES
 ESTIMATED QUANTITIES
 BRIDGE NO. MAH-18-1629
 UNDER VESTAL ROAD
 MAHONING COUNTY STA.

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	LRP		DWP	L.G.H. 9-18-63	

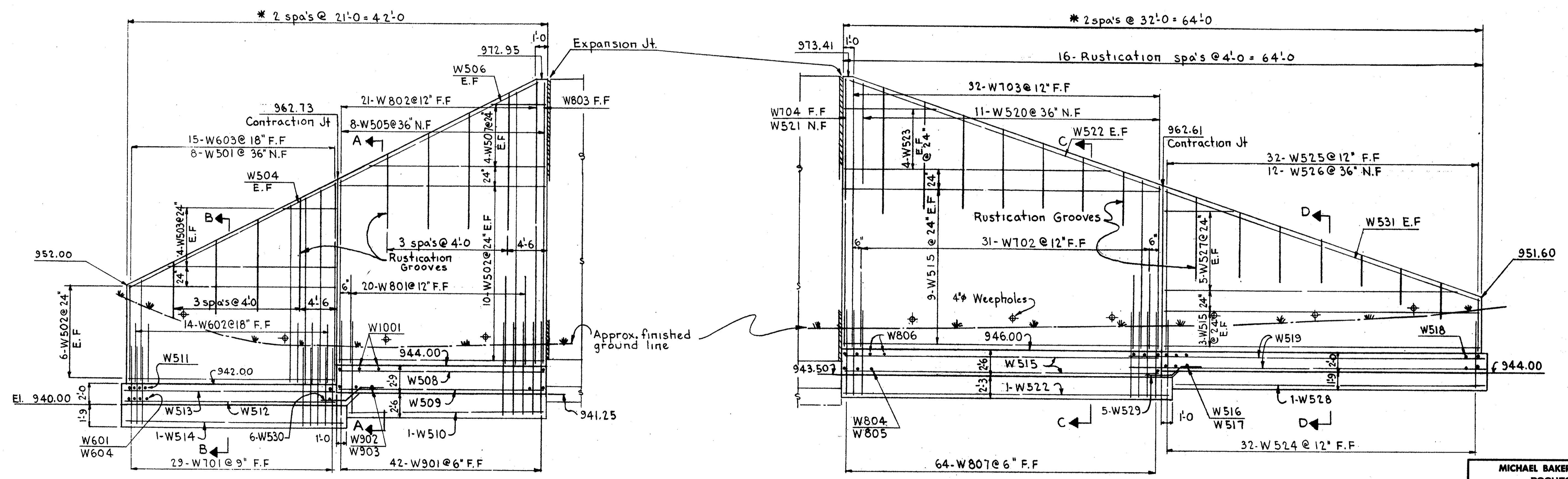
MAHONING COUNTY
MAH-18-15.50



WING - C

PLAN

WING - D



ELEVATION

* Dimensions measured along front face of wall

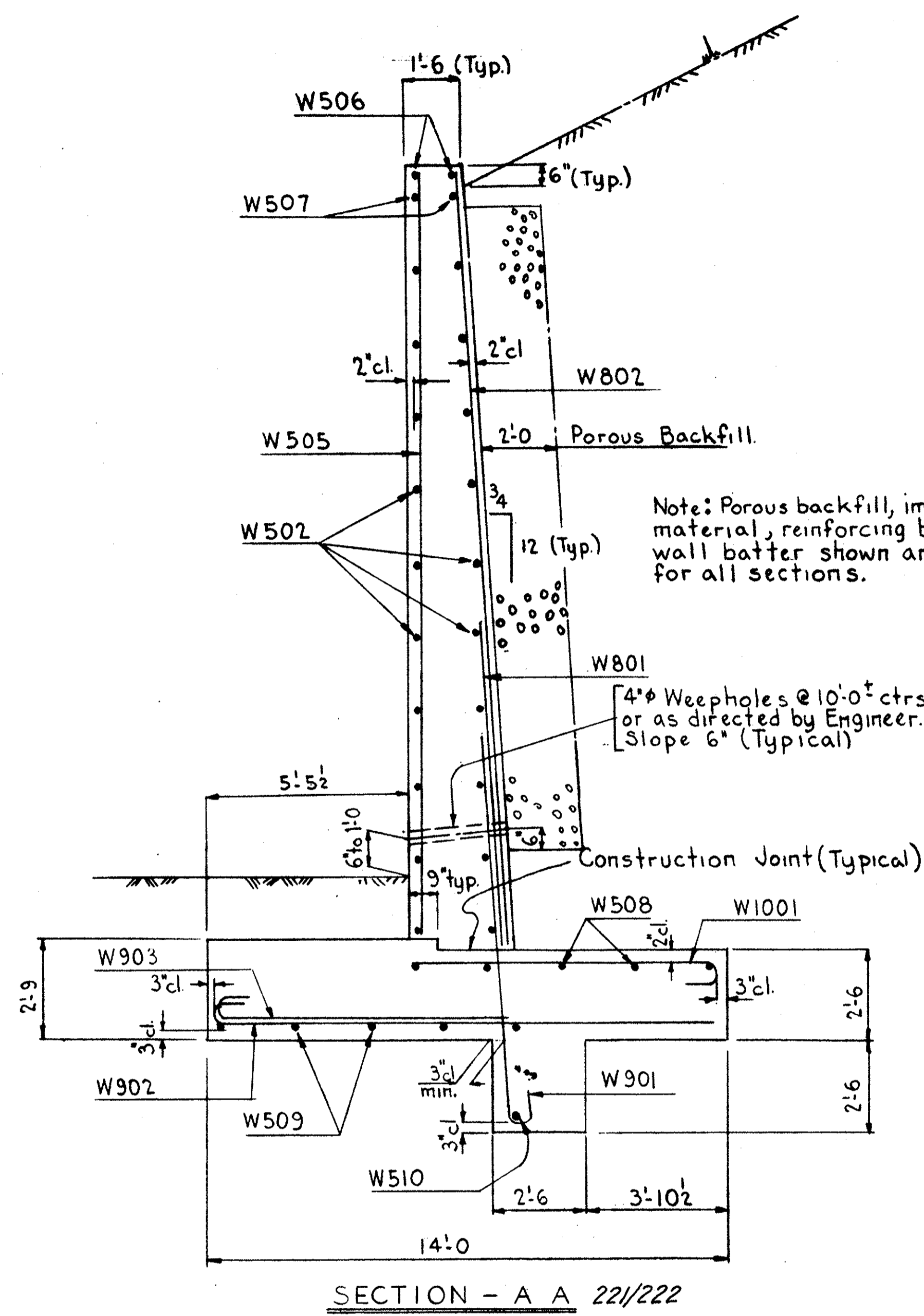
- Note:
- Excavation for shear keys shall be neat
 - N.F = Near Face
 - F.F = Far Face
 - E.F = Each Face
 - For Wing C and Wing D Sections, Contraction and Expansion Joint details, See Sht. #222

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

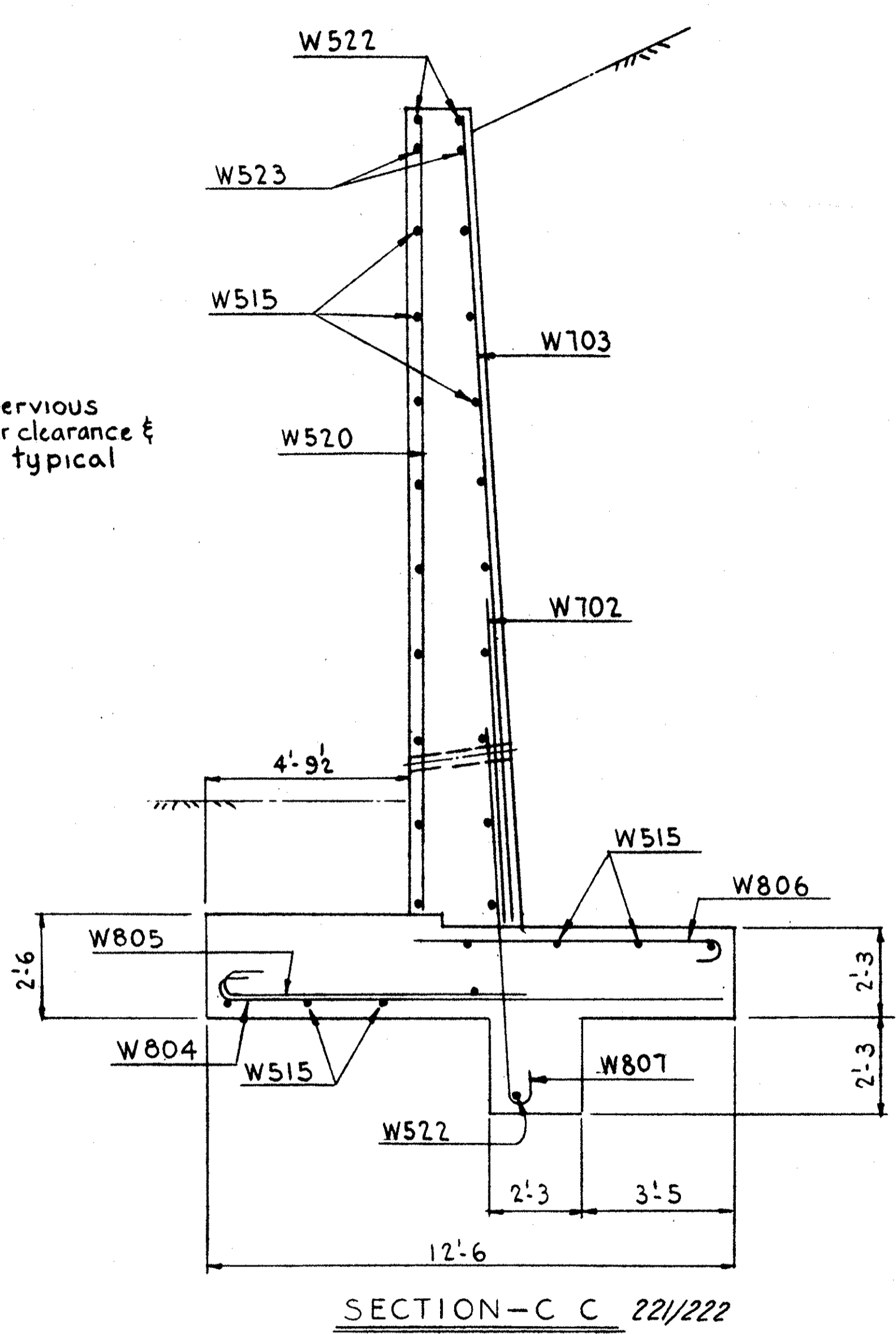
ABUTMENT 1 WINGWALLS
BRIDGE NO MAH-18-1629
UNDER VESTAL ROAD

MAHONING CO. STA 559+80.95

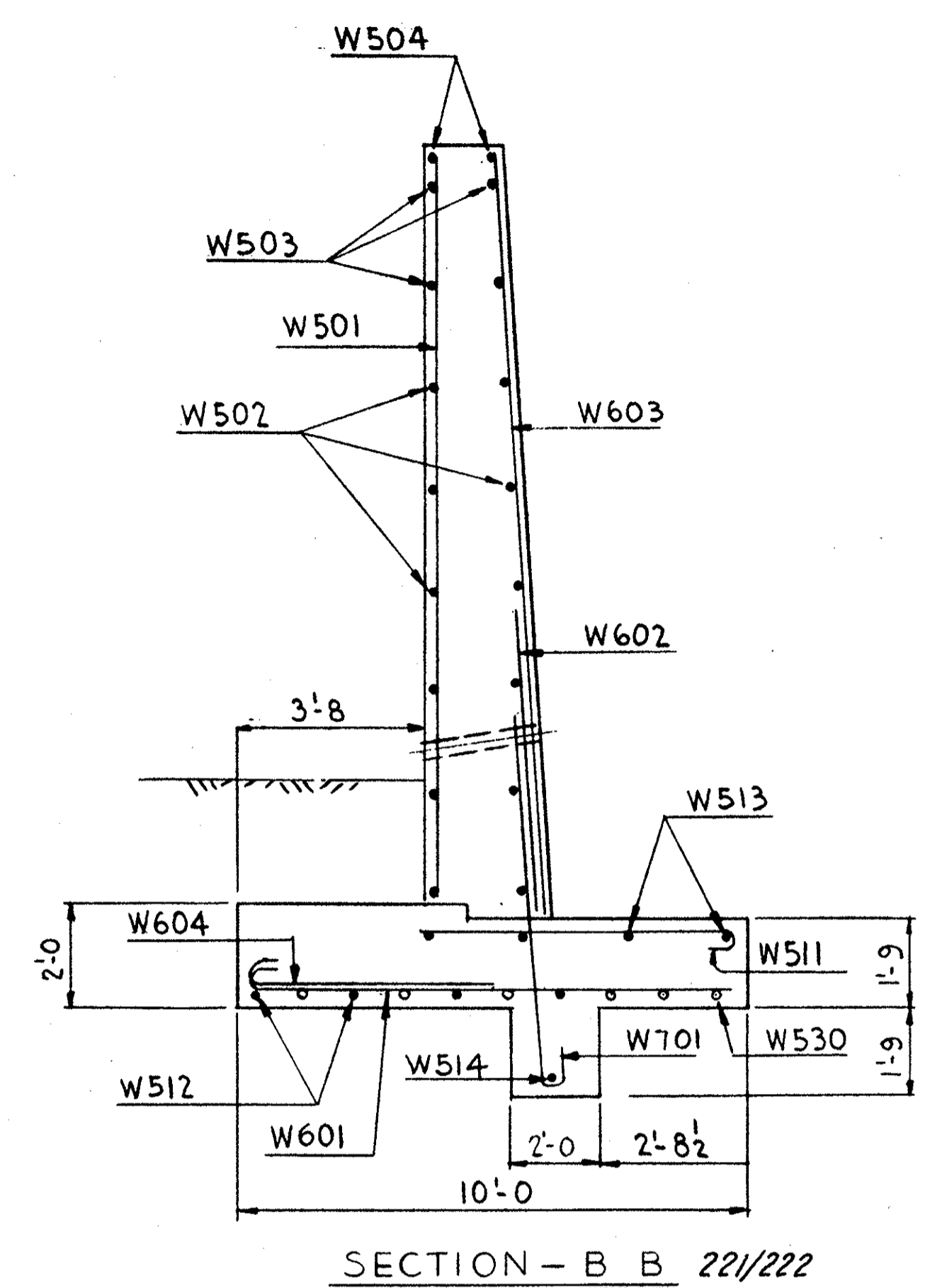
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
S.J.F.	bo		LRD	L.G.H. 9-18-63	



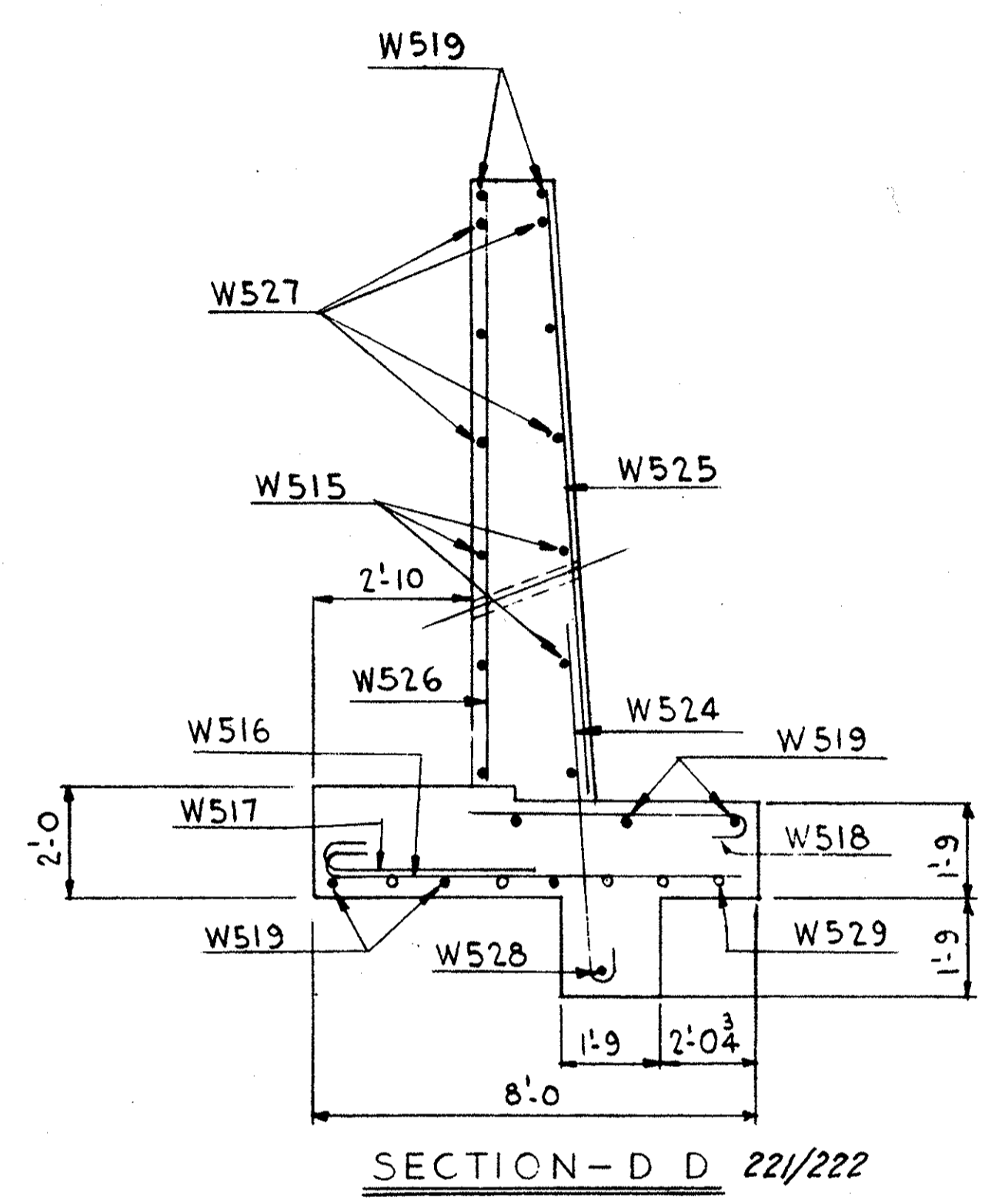
SECTION - A A 221/222



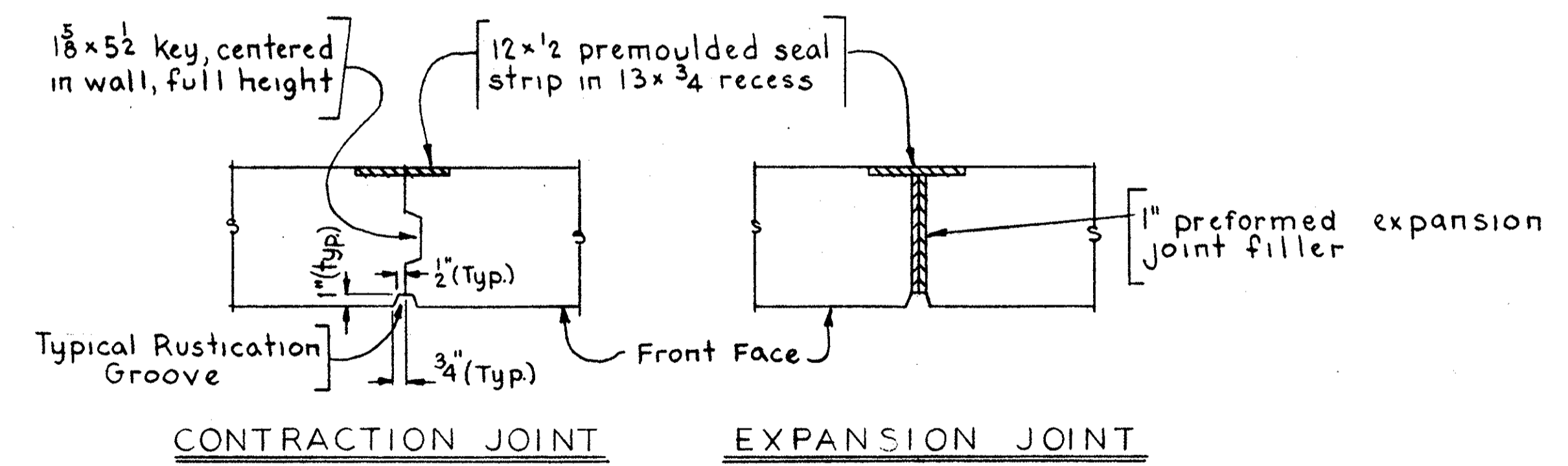
SECTION - C C 221/222



SECTION - B B 221/222



SECTION - D D 221/222



CONTRACTION JOINT

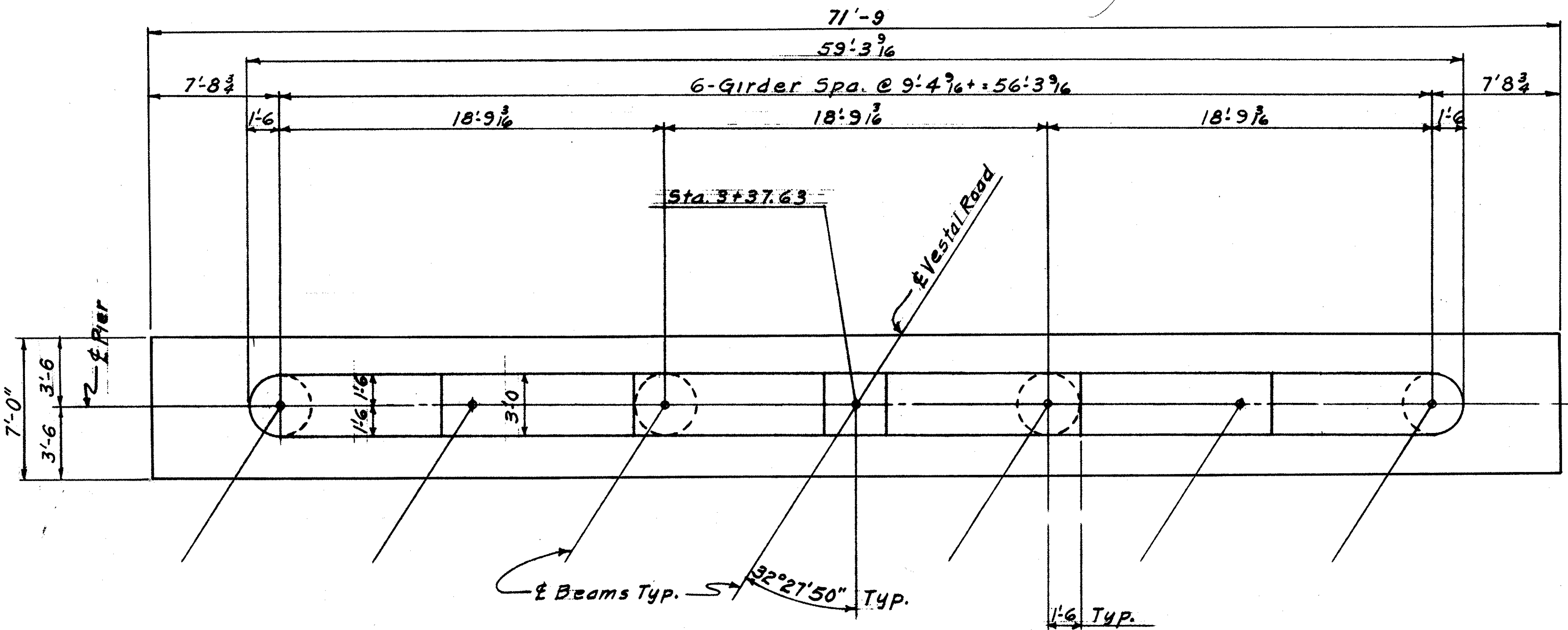
EXPANSION JOINT

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT 1 WINGWALLS BRIDGE NO. MAH-18-1629 UNDER VESTAL ROAD					
MAHONING CO. STA 559+80.95					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
S.J.F.	bo		LRD	L.G.H. 9-18-63	

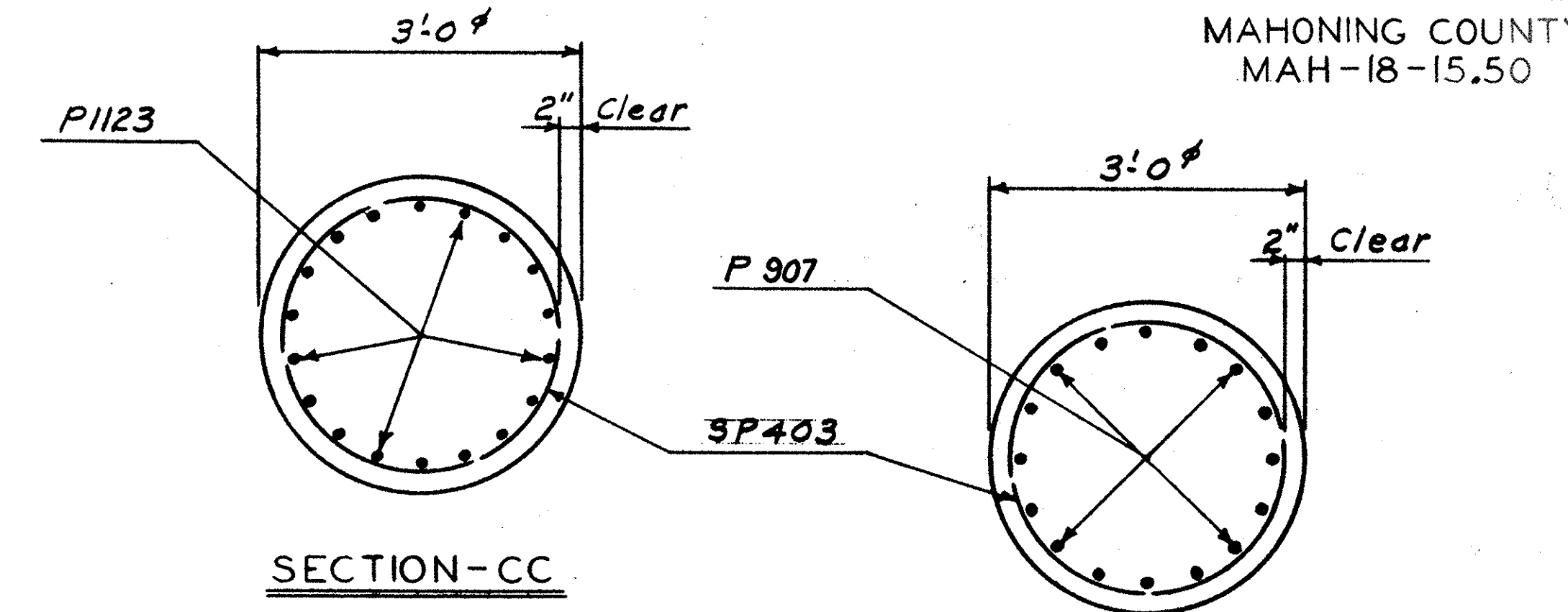
FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

223

MAHONING COUNTY
MAH-18-15.50

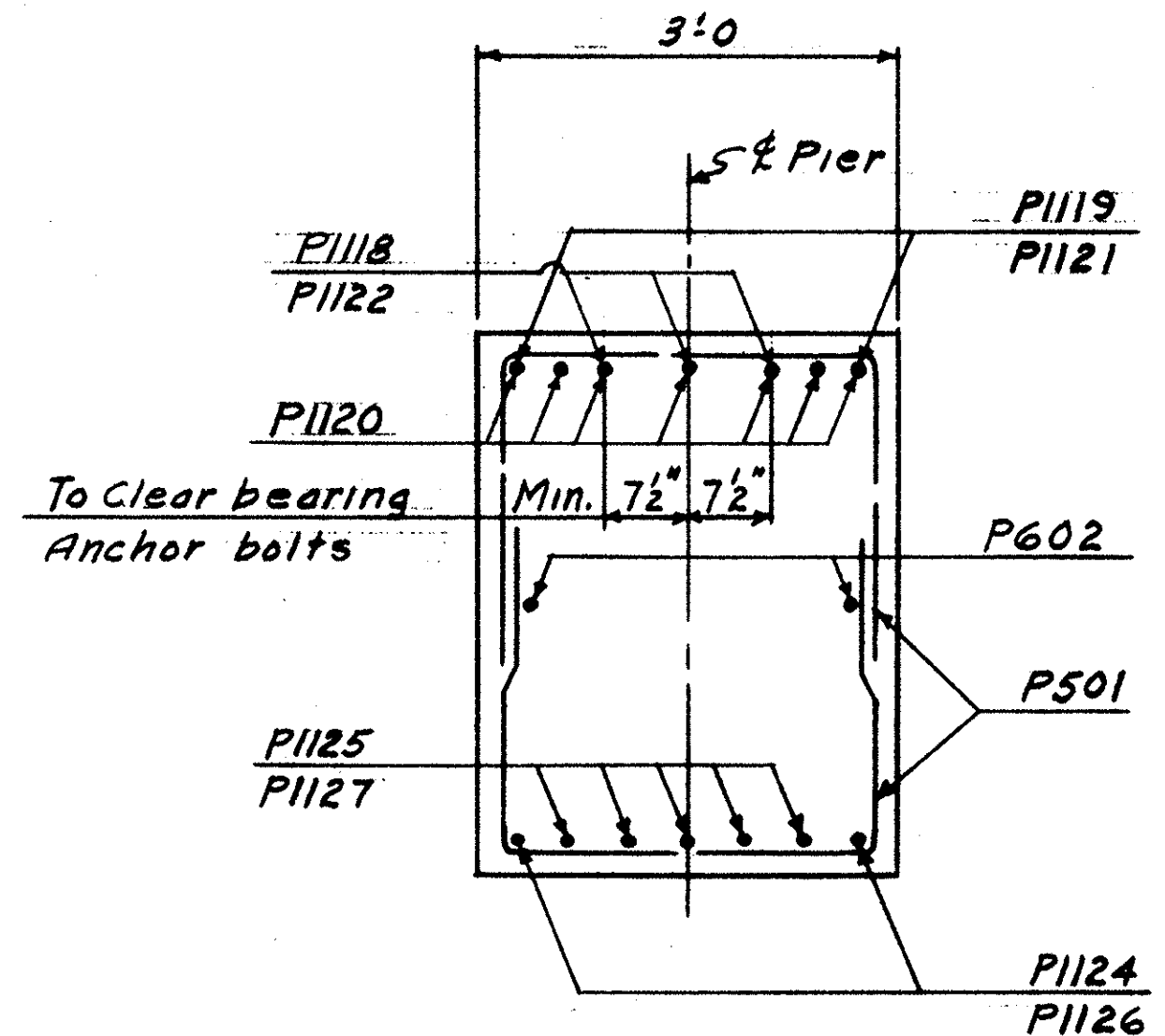


PLAN PIER 1

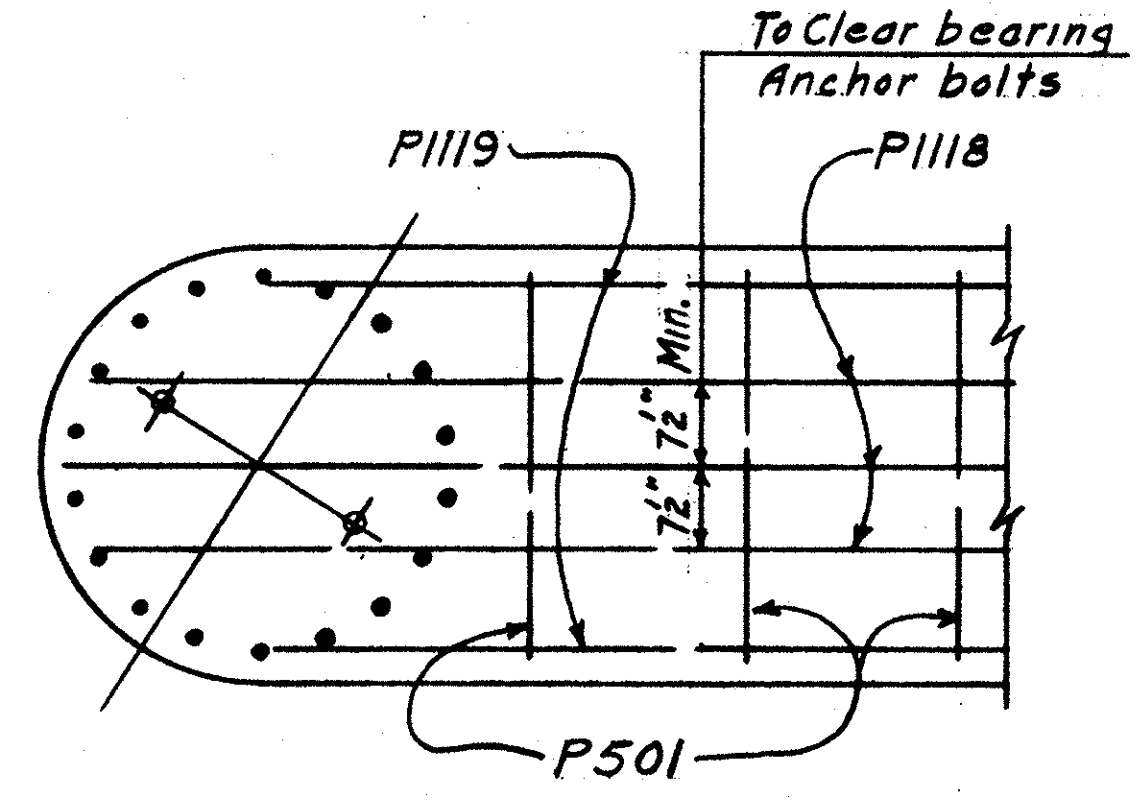


SECTION-CC

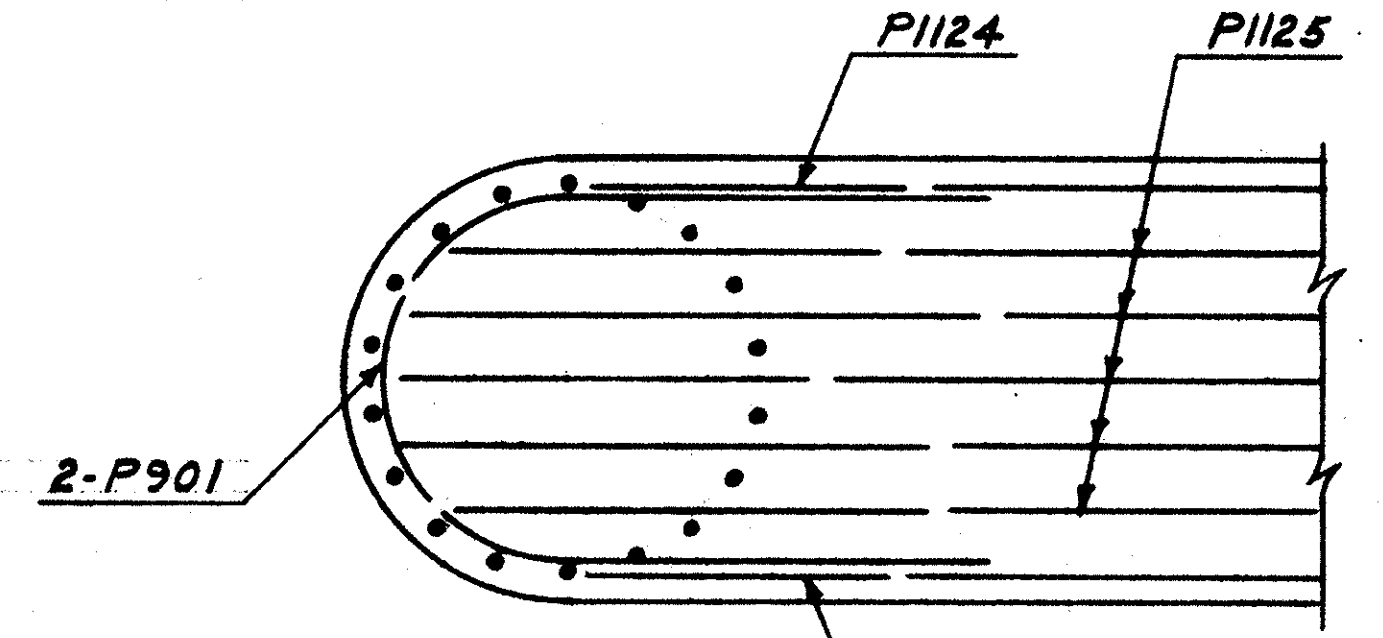
SECTION-EE



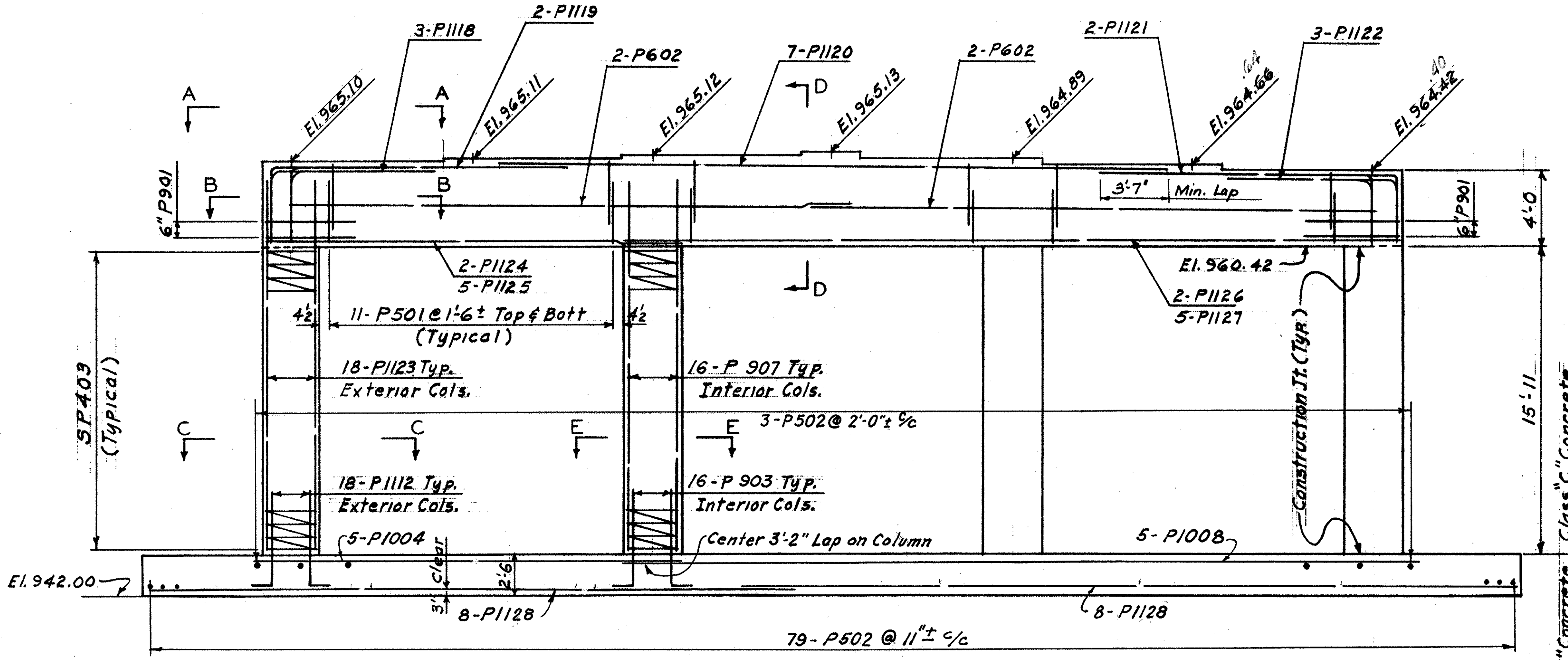
SECTION-DD



SECTION-AA



SECTION-BB

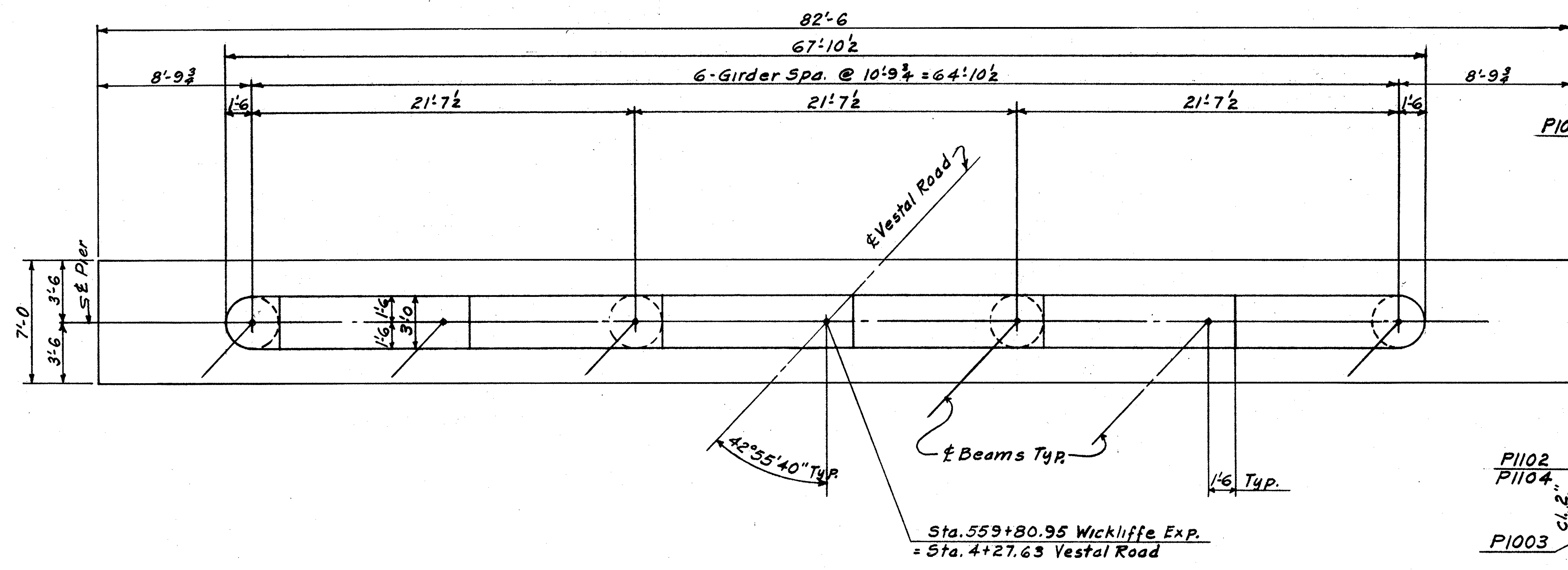


ELEVATION

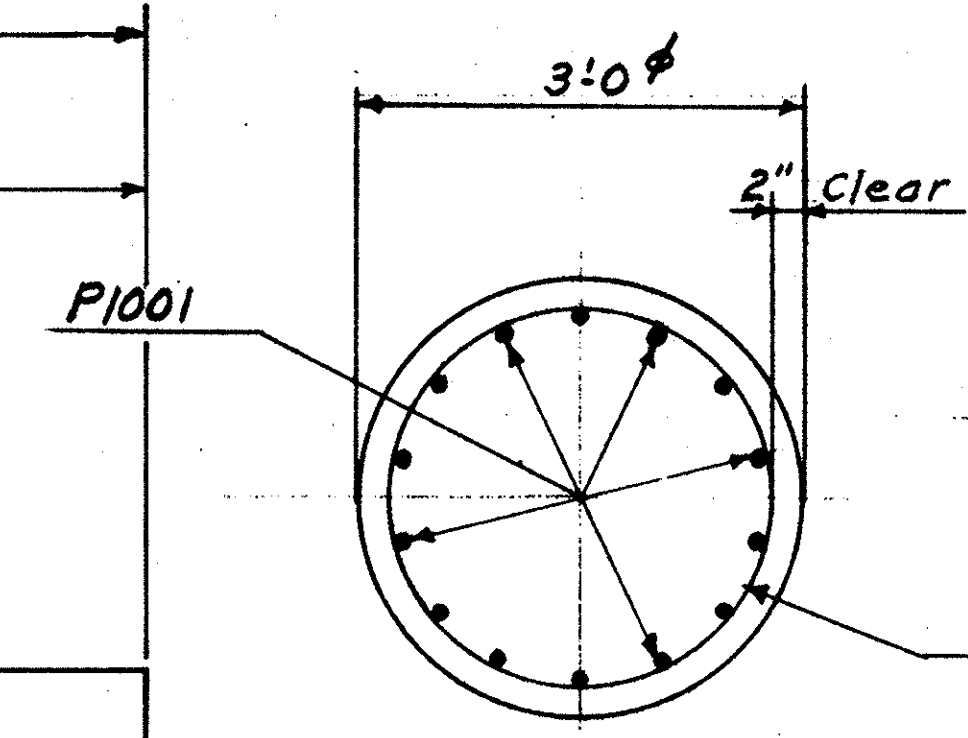
Class "C" Concrete

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
PIER NO. 1 BRIDGE NO. MAH-18-1629 UNDER VESTAL ROAD					
MAHONING CO. Sta. 559+80.95					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	TP		DWP	L.G.H. 9-18-63	12-22-65

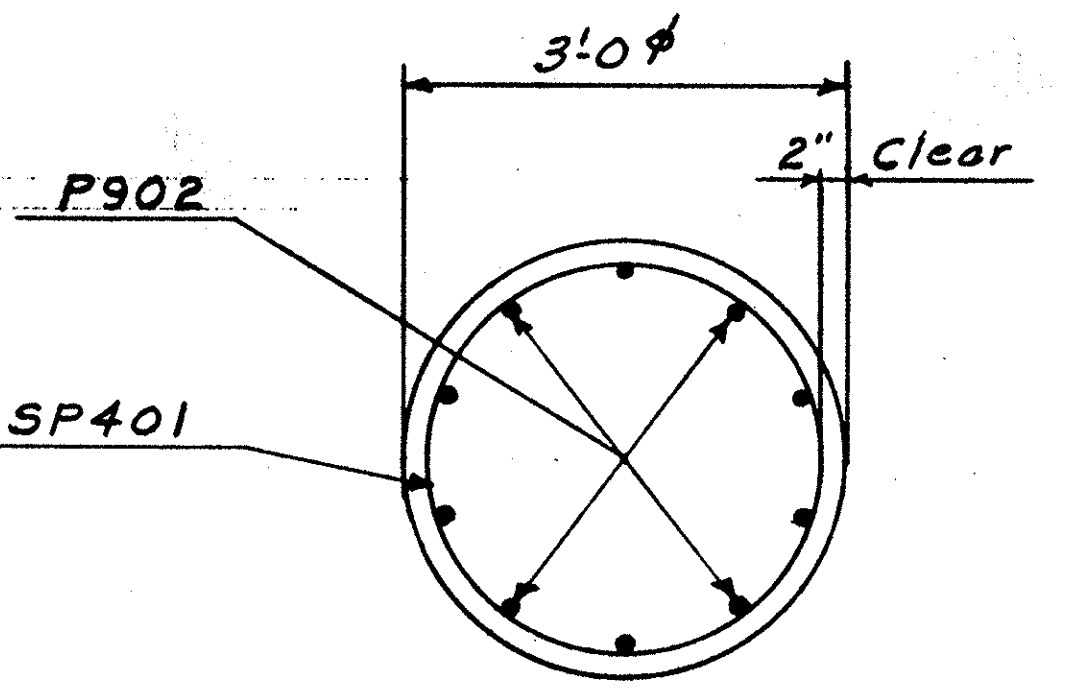
MAHONING COUNTY
MAH-18-15.50



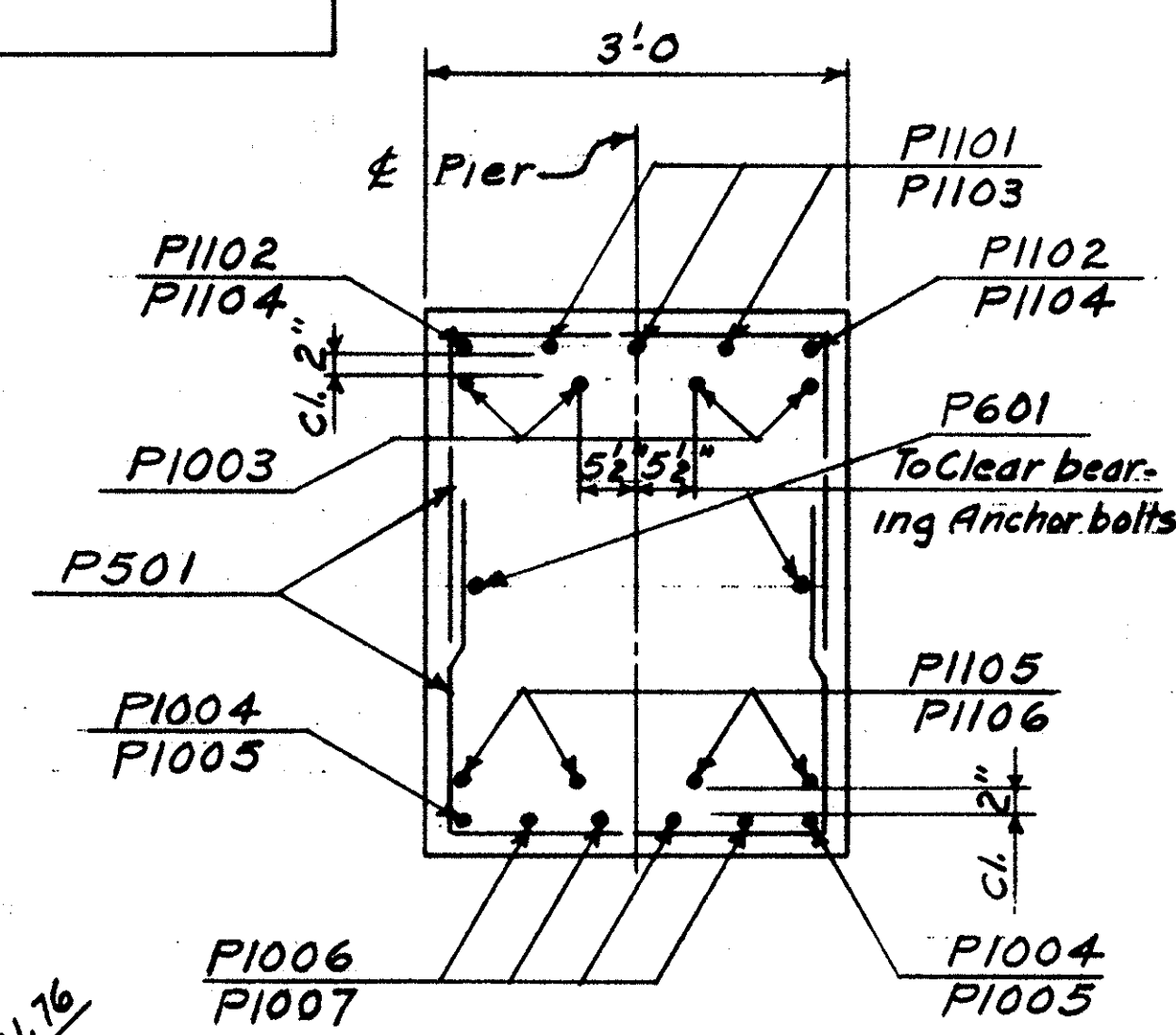
PLAN PIER 2



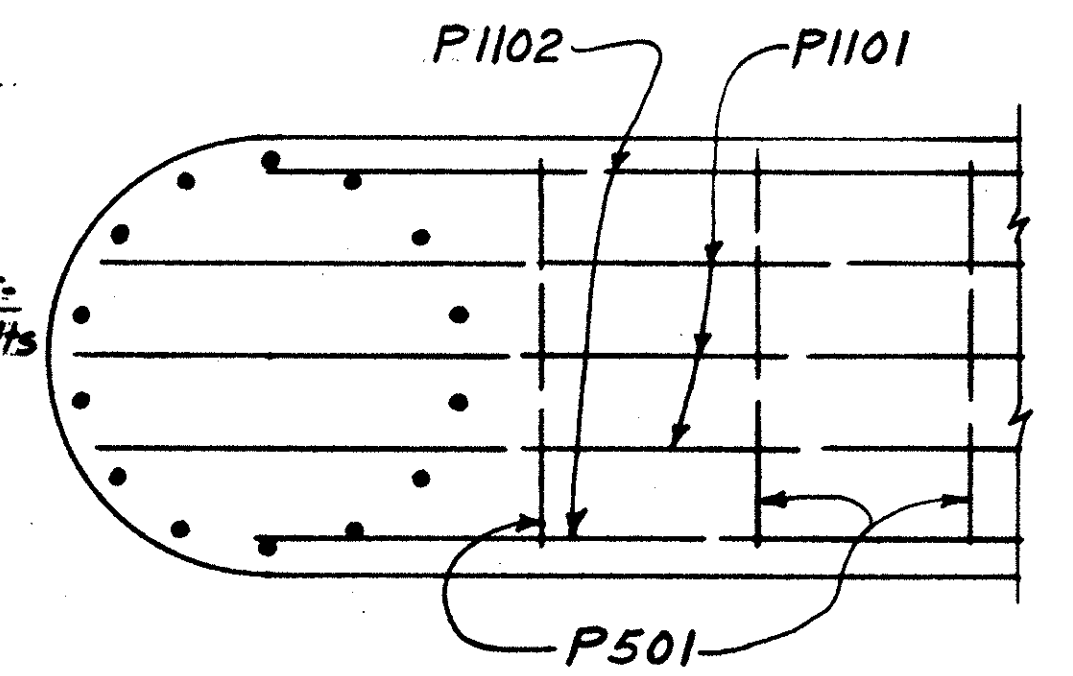
SECTION-CC



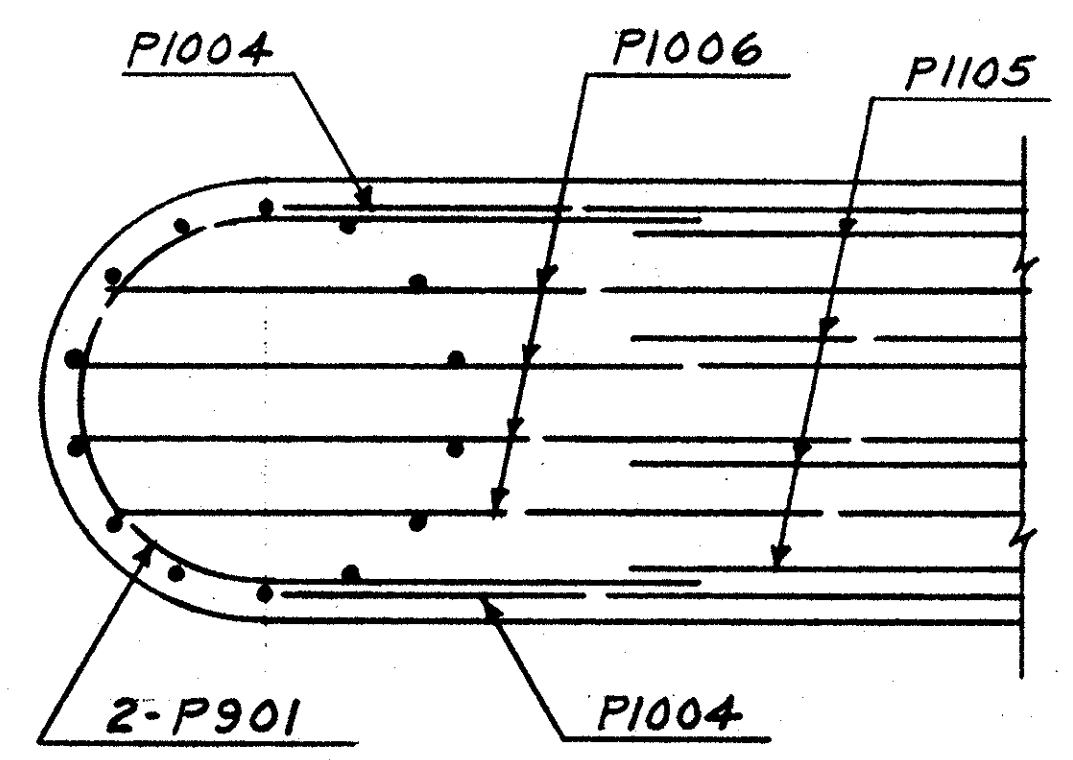
SECTION-EE



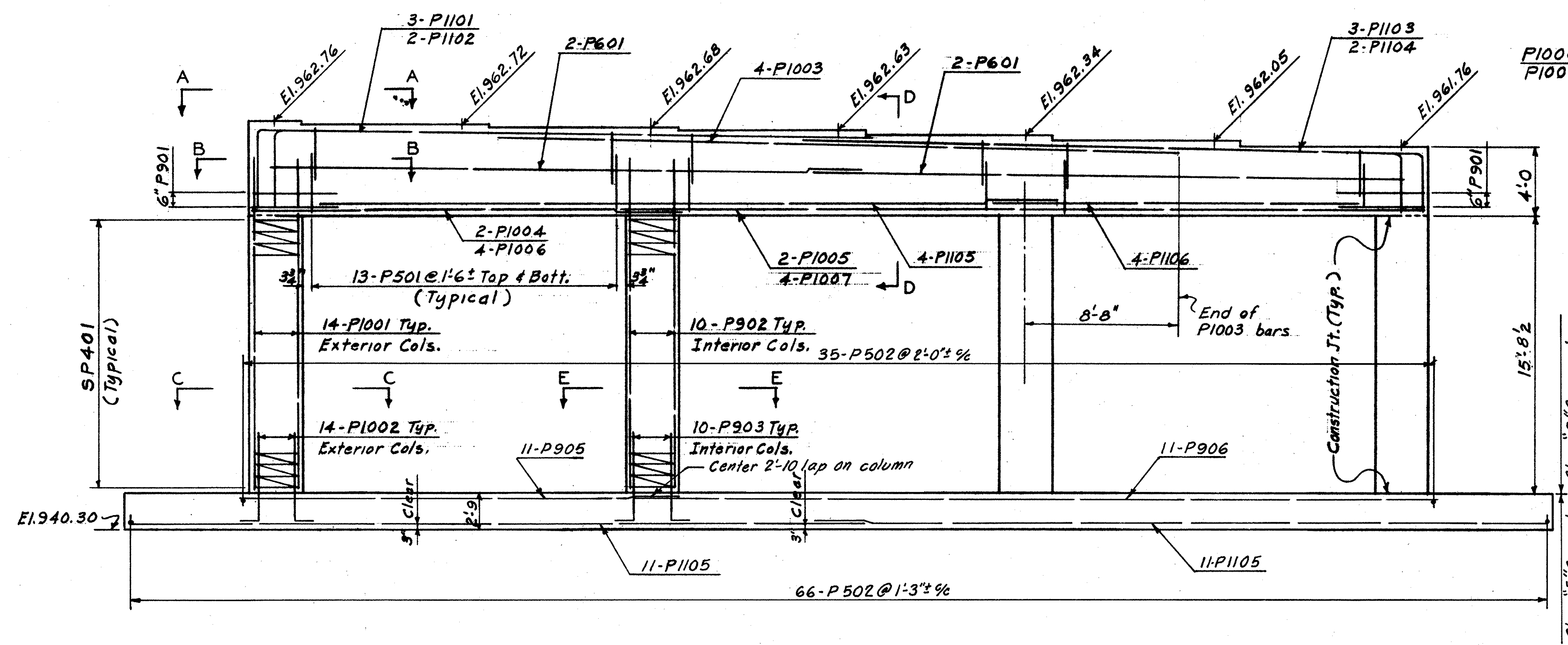
SECTION-DD



SECTION-AA



SECTION-BB



ELEVATION

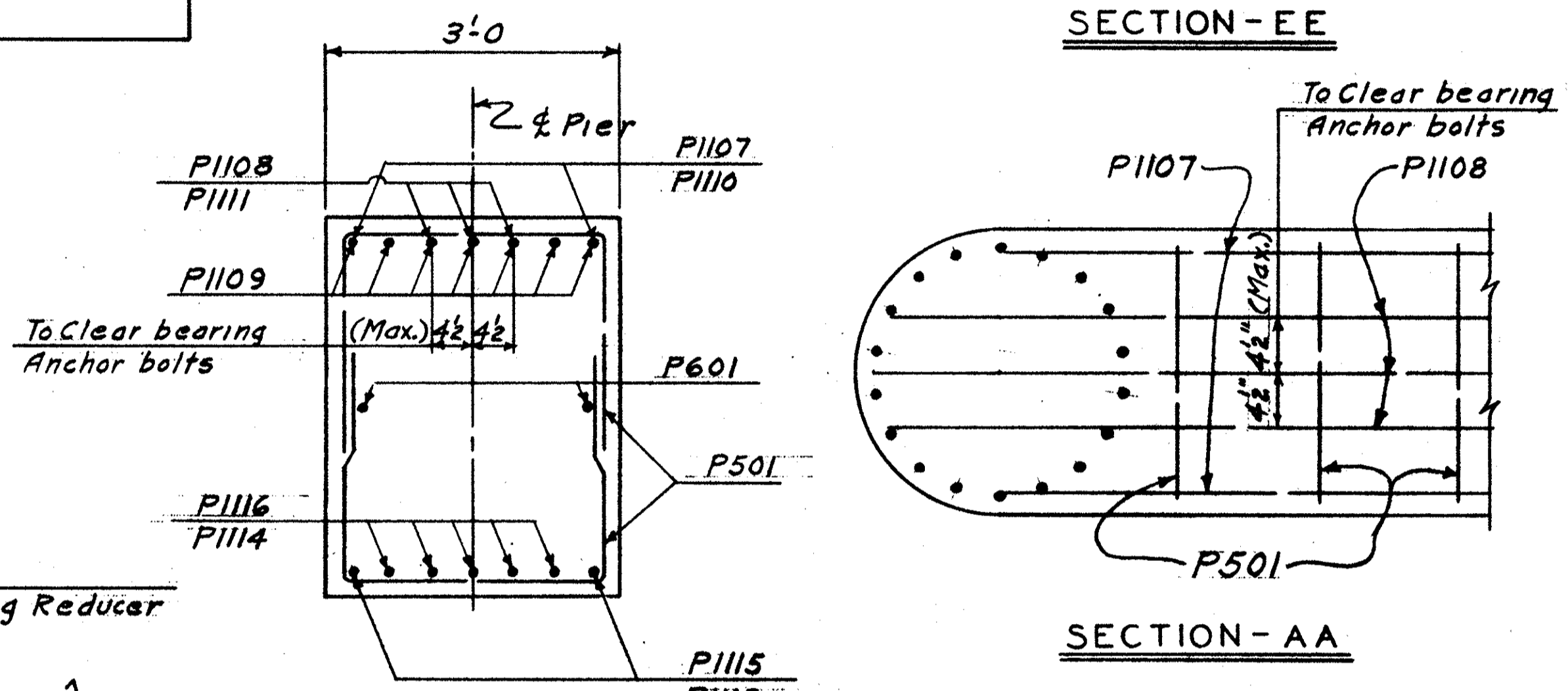
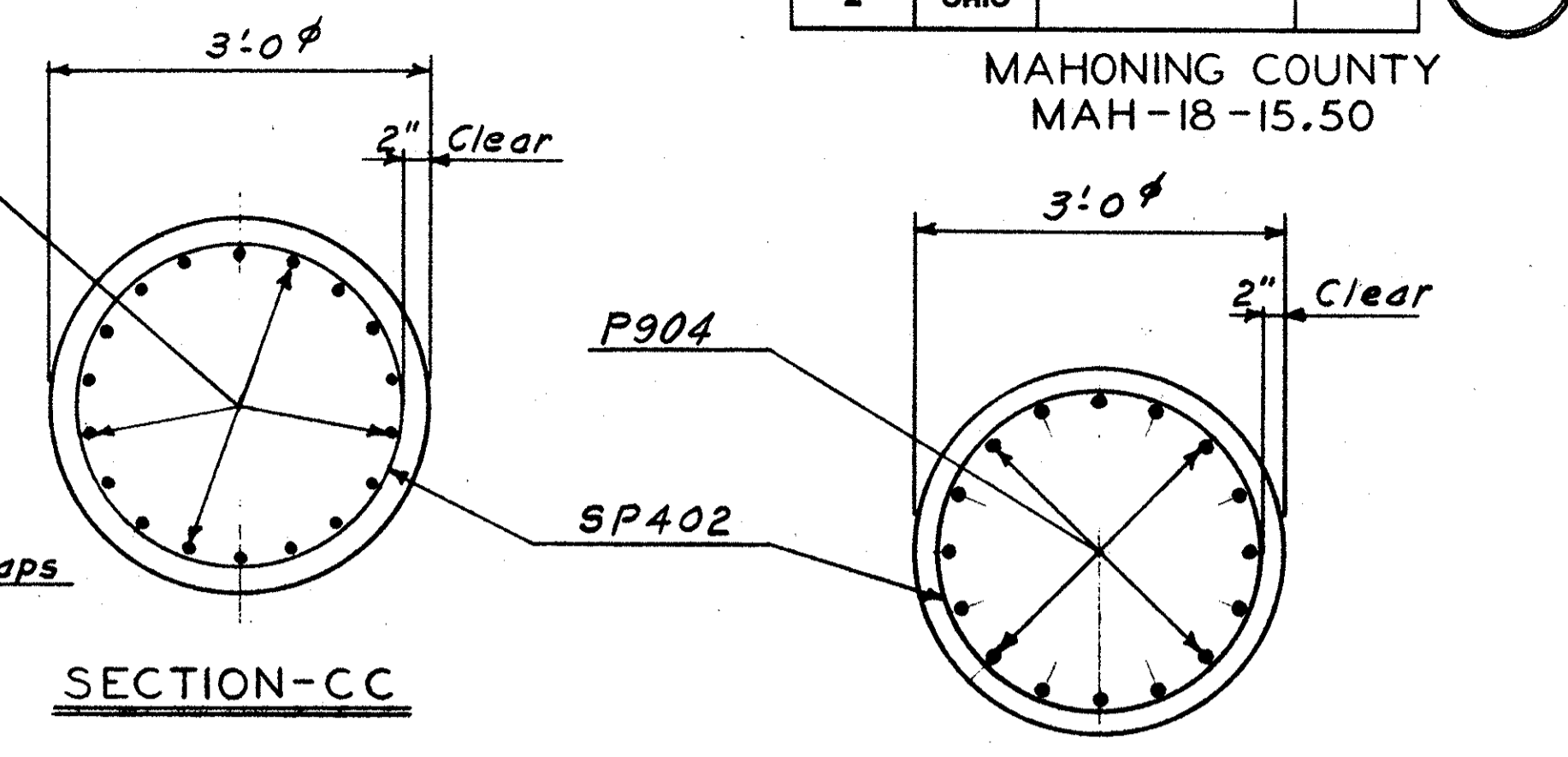
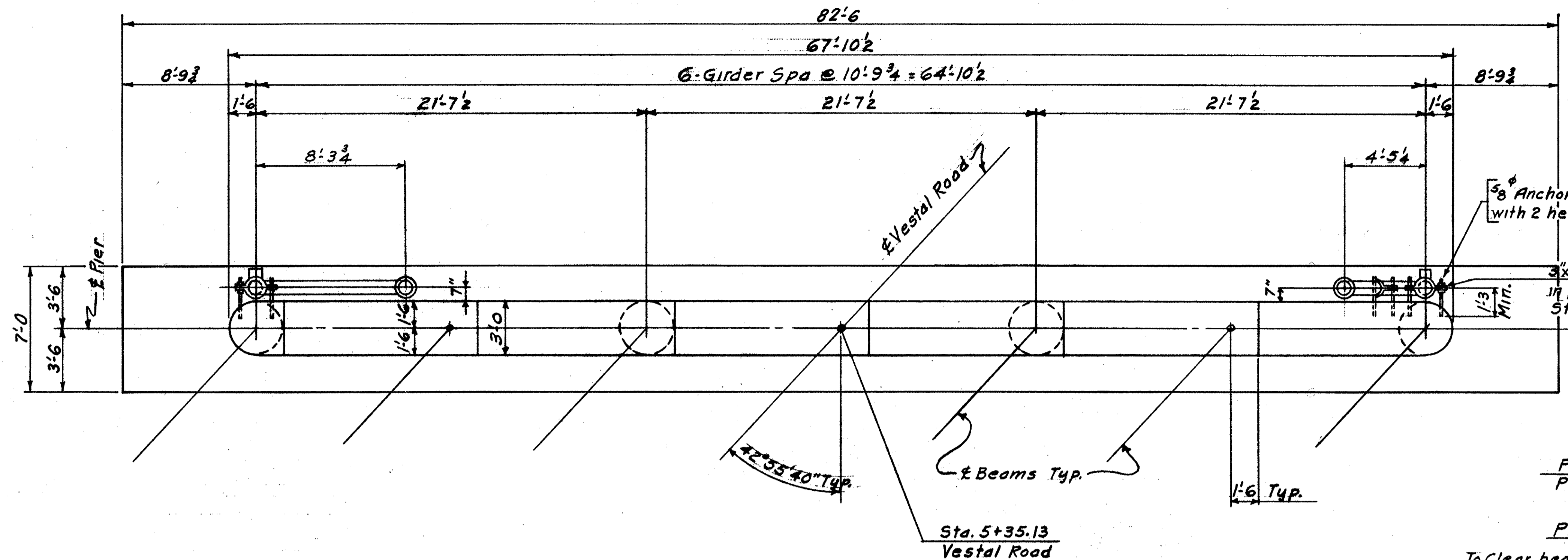
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

PIER NO. 2
BRIDGE NO. MAH-18-1629
UNDER VESTAL ROAD

MAHONING CO. Sta. 559+80.95

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	TP		DWP	LGH 9-18-63	

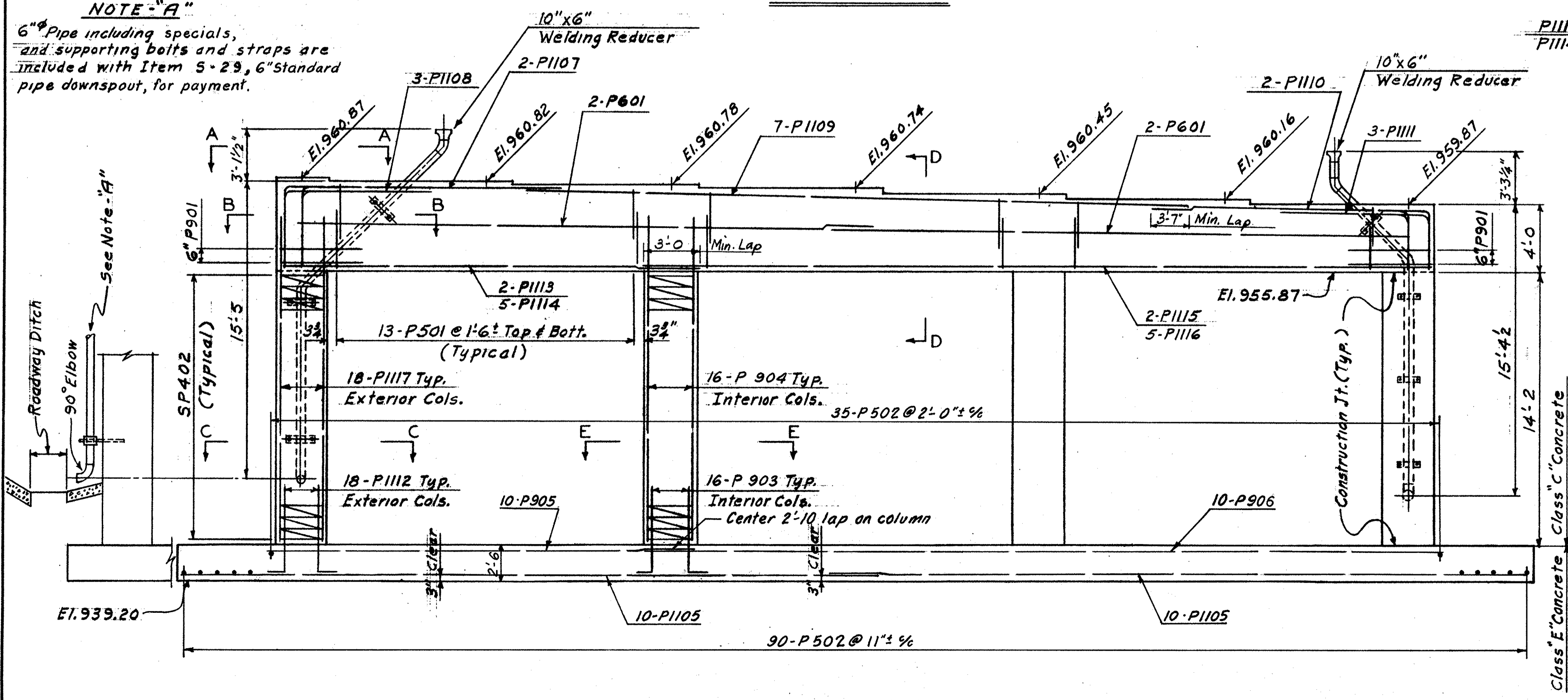
MAHONING COUNTY
MAH-18-15.50



NOTE "A"

6" Pipe including specials, and supporting bolts and straps are included with Item 5-29, 6" Standard pipe downspout, for payment.

PLAN PIER 3



ELEVATION

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

PIER NO. 3
BRIDGE NO. MAH-18-1629
UNDER VESTAL ROAD

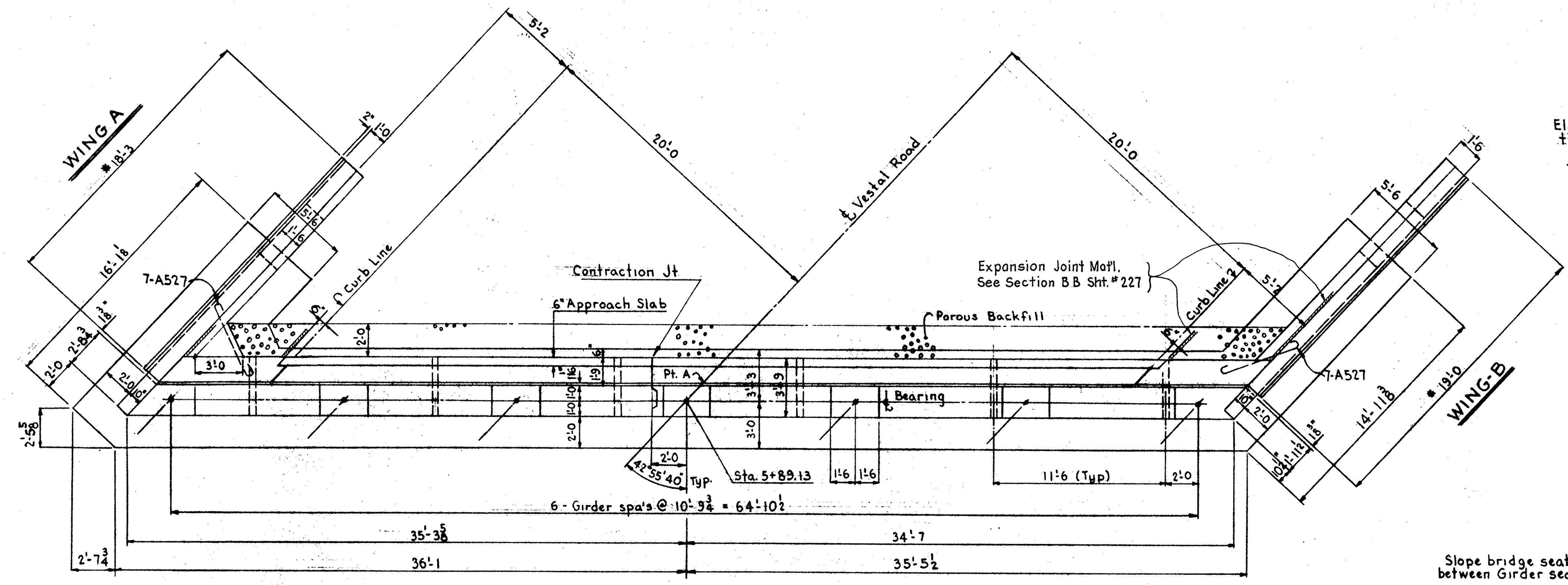
MAHONING CO. Sta. 559+80.95

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
SJF	TP		DWP	1.6.4.	9-18-63

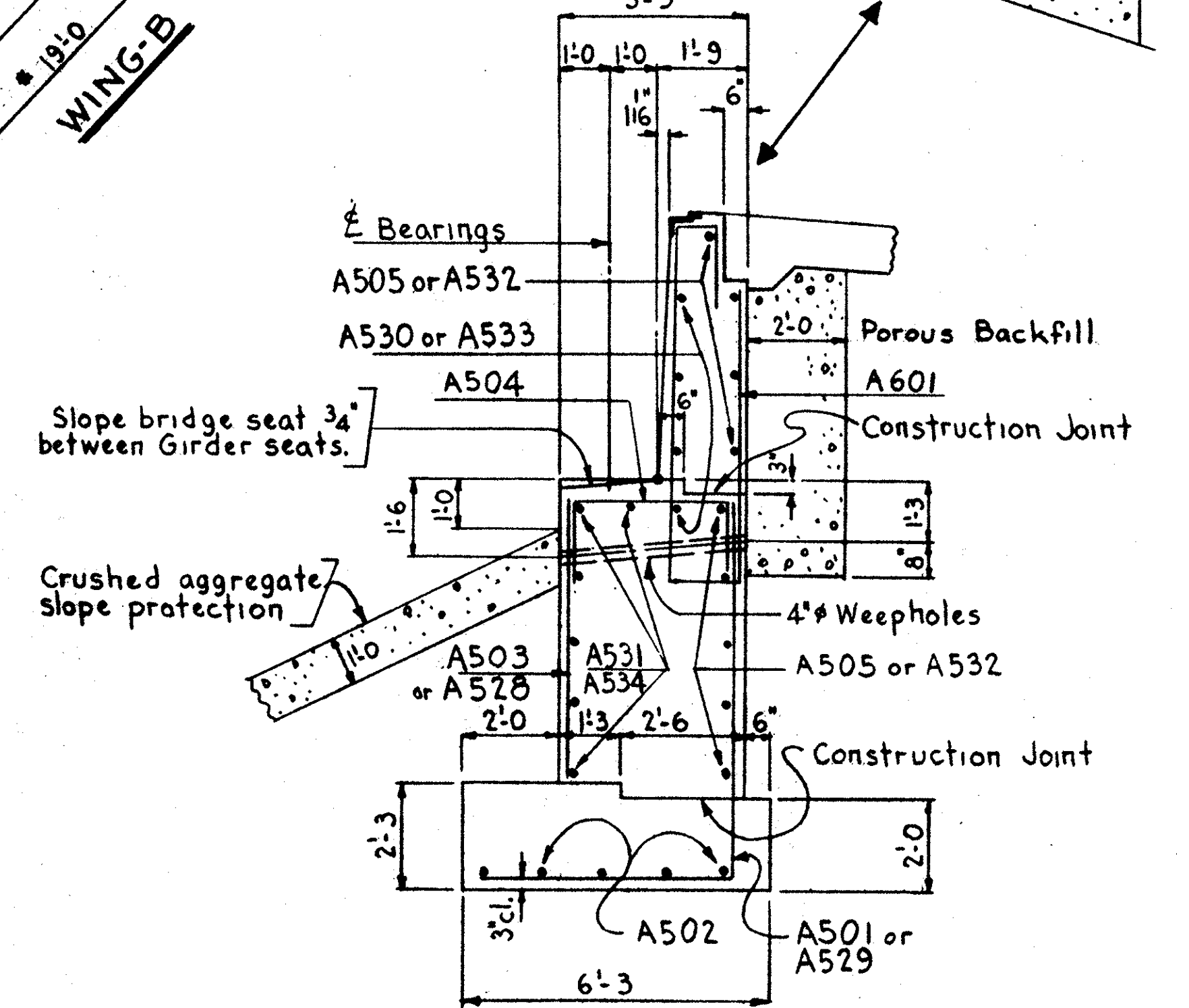
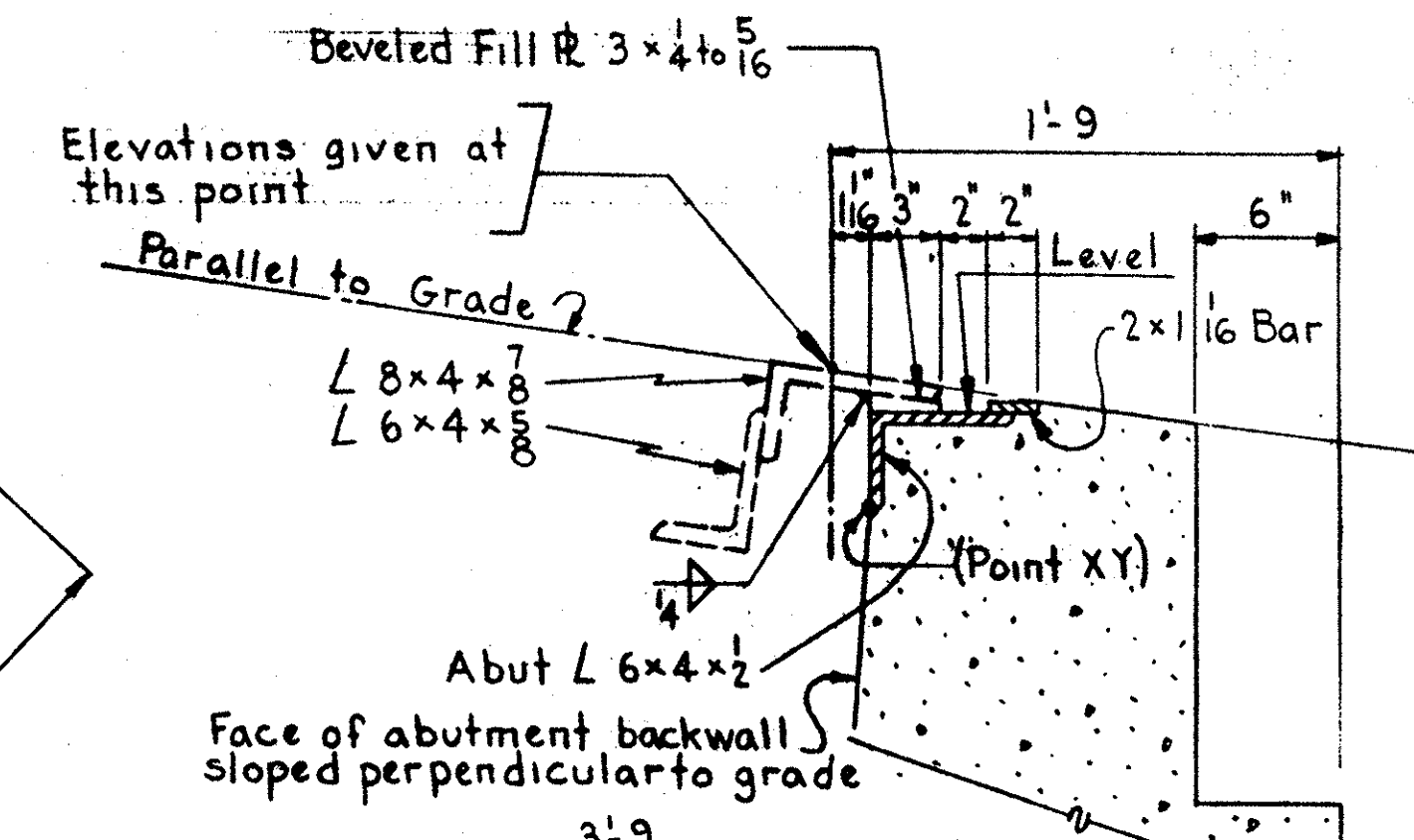
FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS
2	OHIO		

226

MAHONING COUNTY
MAH-18-15.50

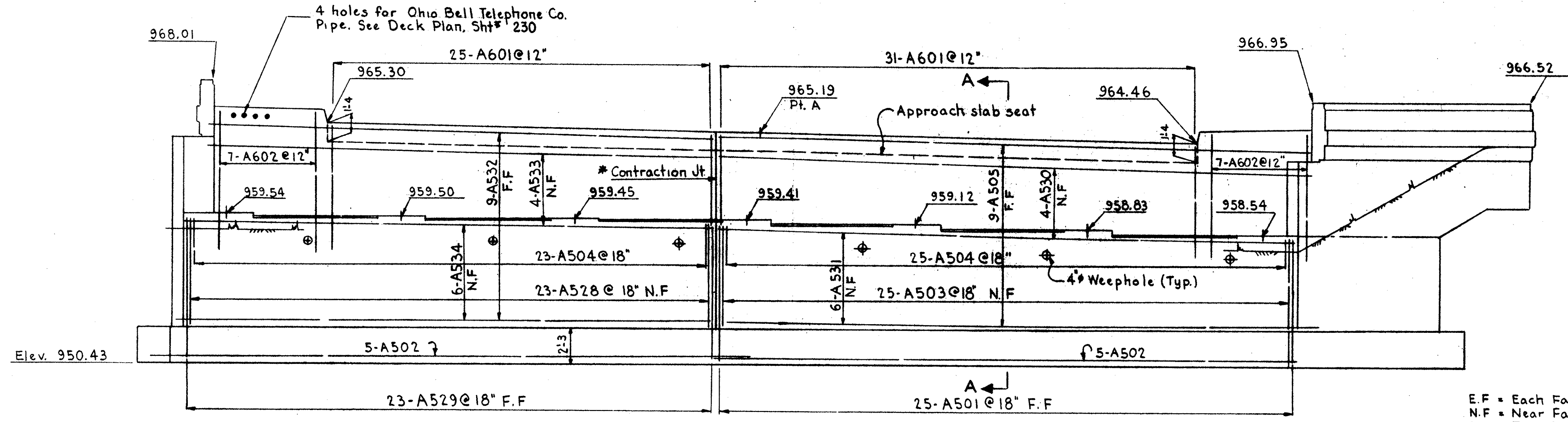


PLAN



SECTION-A A

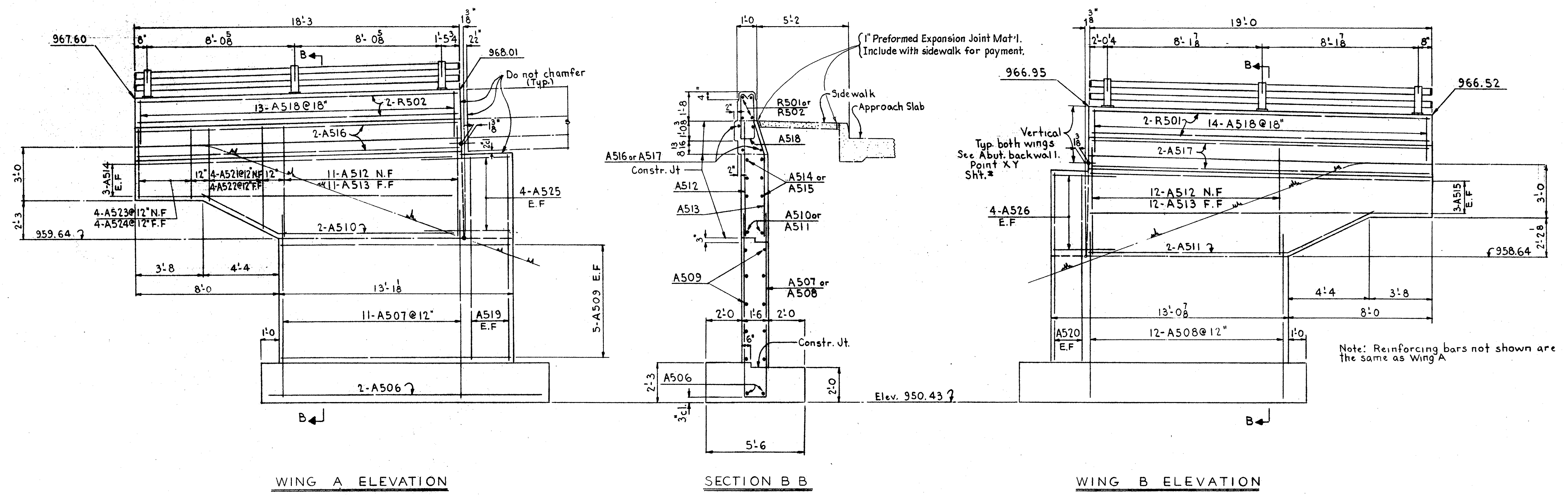
* For Contraction Joint Detail, See Sh't # 222
12x2 seal strip at Contraction Joint shall be from top of footing to approach slab seat.



ELEVATION

E.F. = Each Face
N.F. = Near Face
F.F. = Far Face

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT 2 DETAILS BRIDGE NO. MAH-18-1629 UNDER VESTAL ROAD					
MAHONING COUNTY STA. 559+80.95					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	bo		LRD	L.G.H. 9-18-63	



N.F = Near Face
E.F = Each Face
F.F = Far Face

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

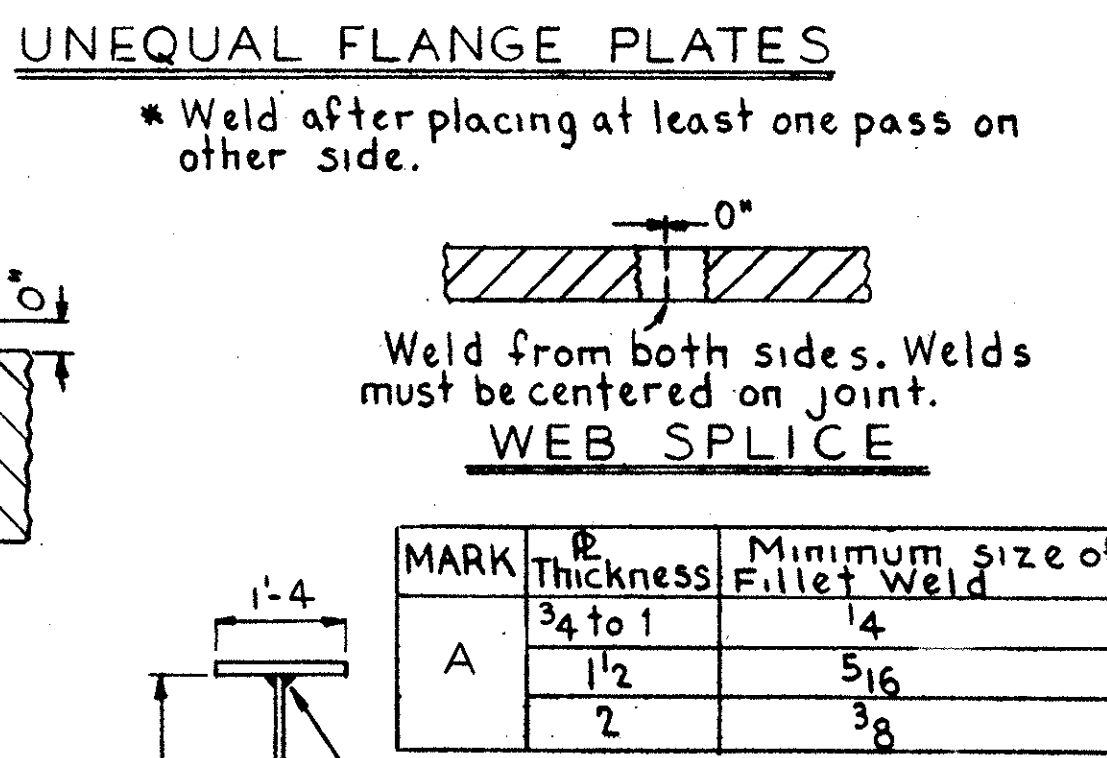
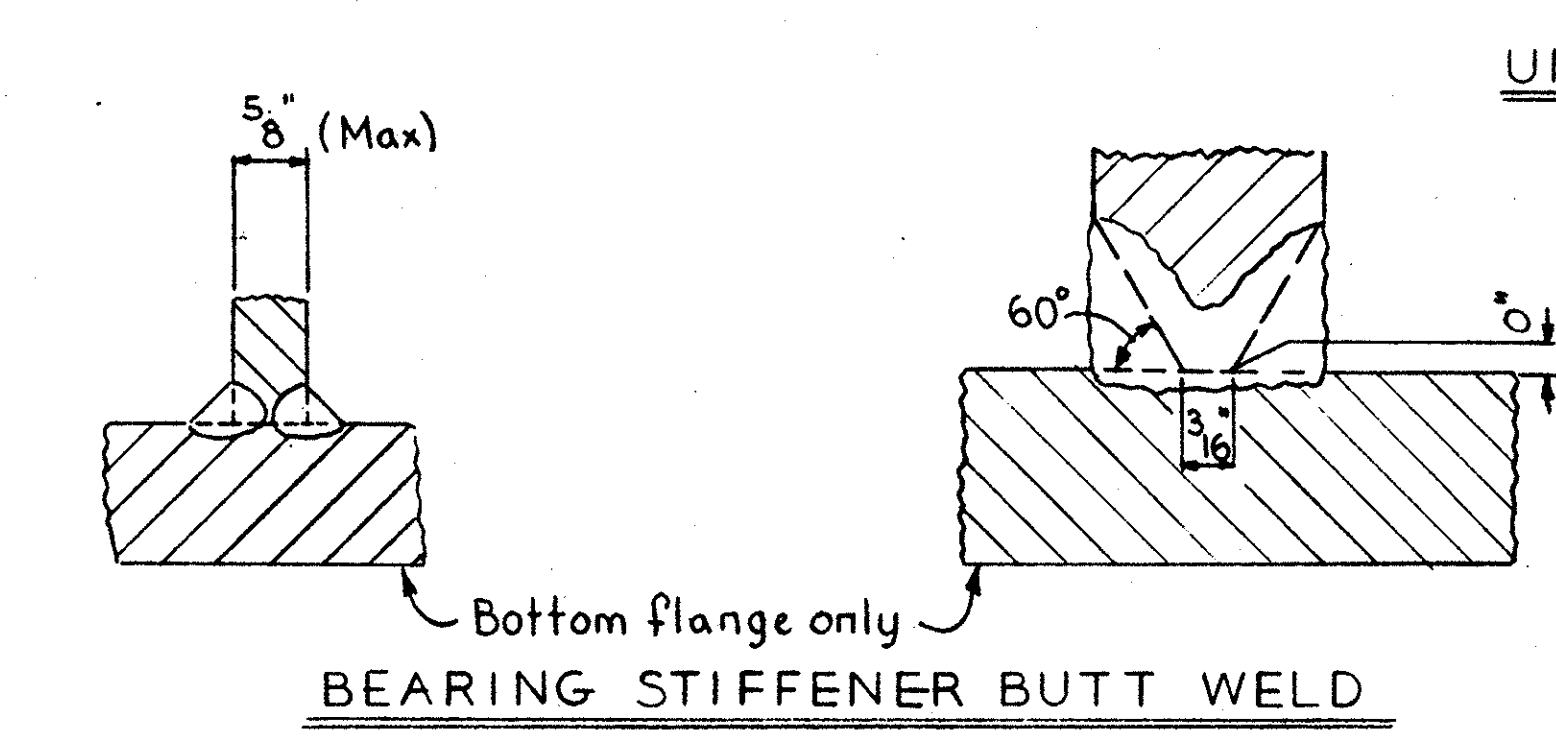
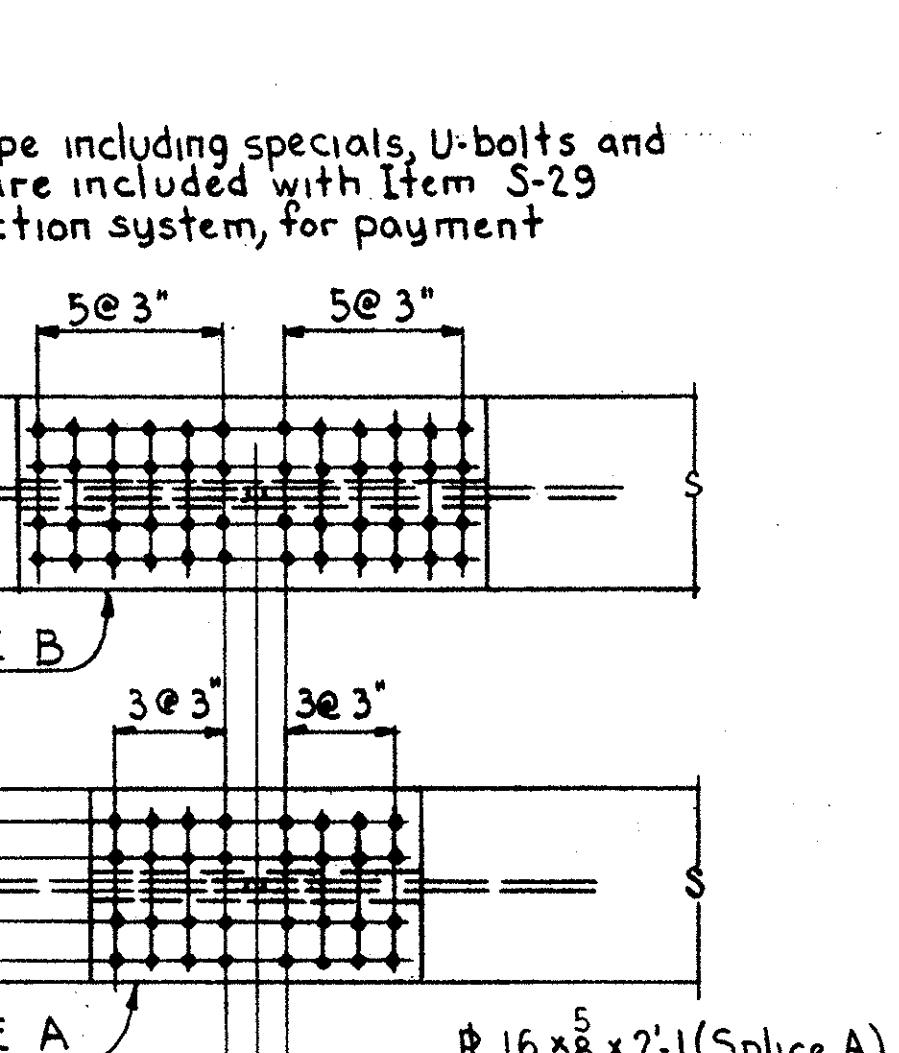
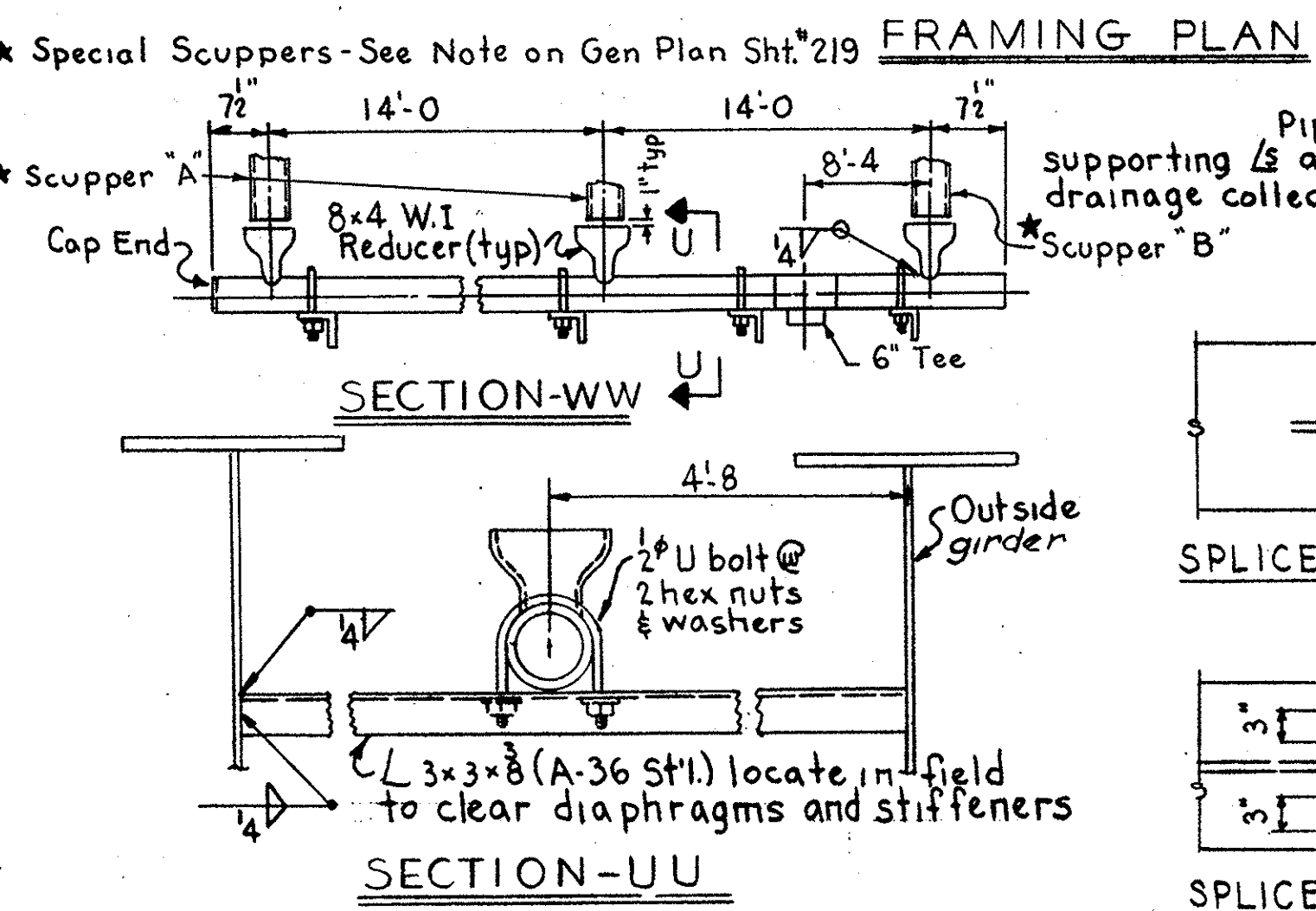
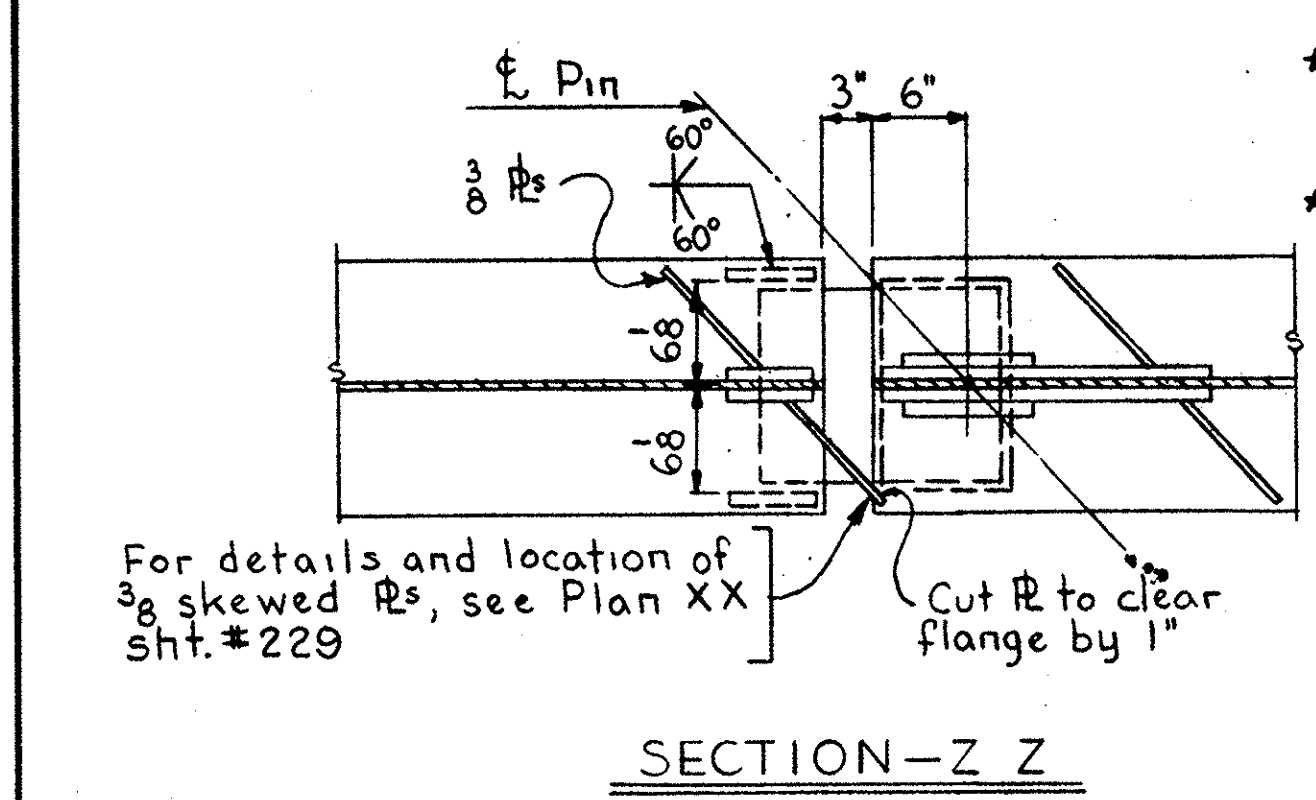
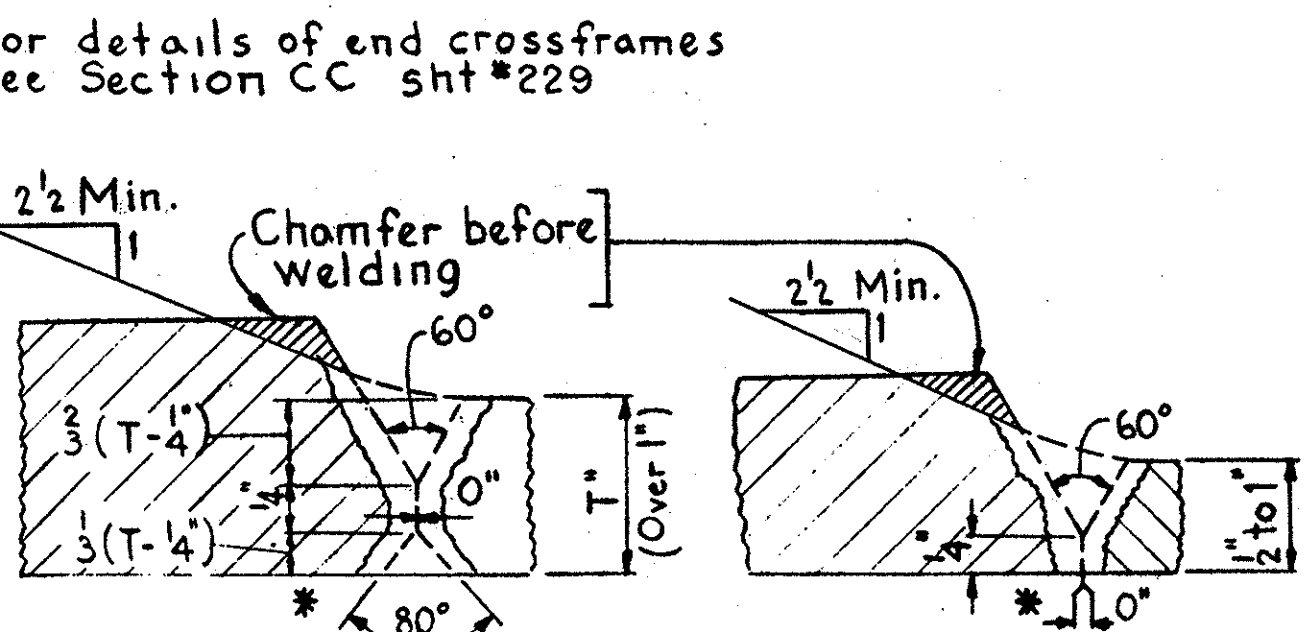
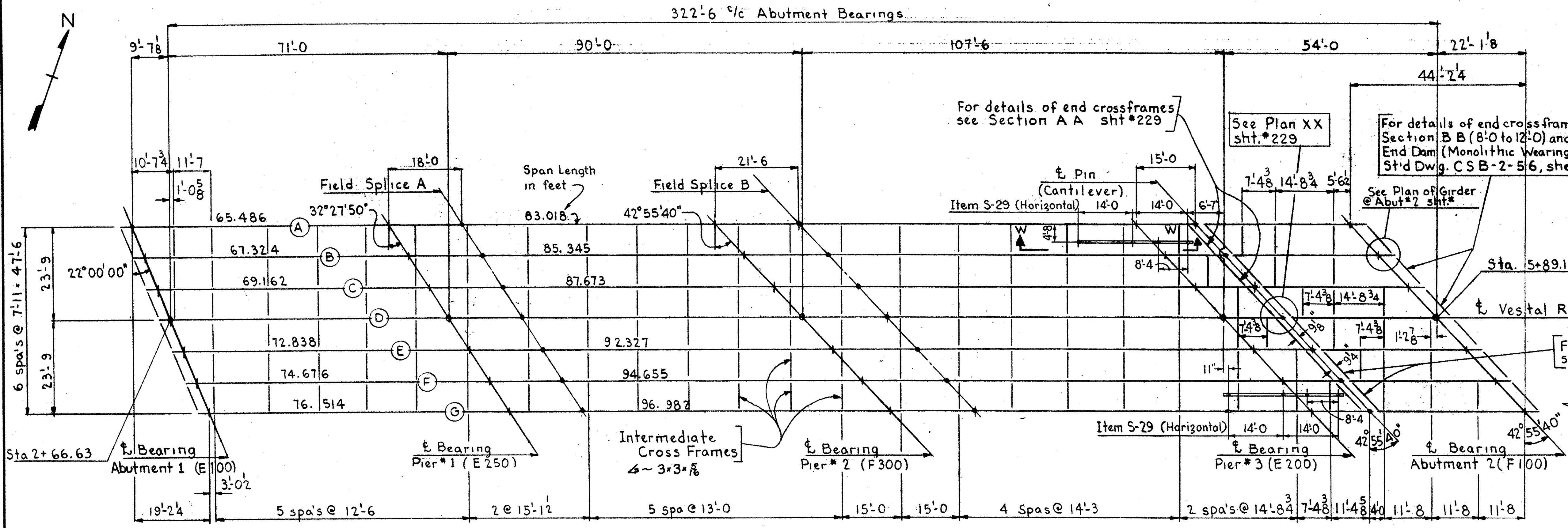
ABUTMENT 2 WINGWALLS
BRIDGE NO. MAH-18-1629
UNDER VESTAL ROAD

MAHONING COUNTY STA. 559+80.95

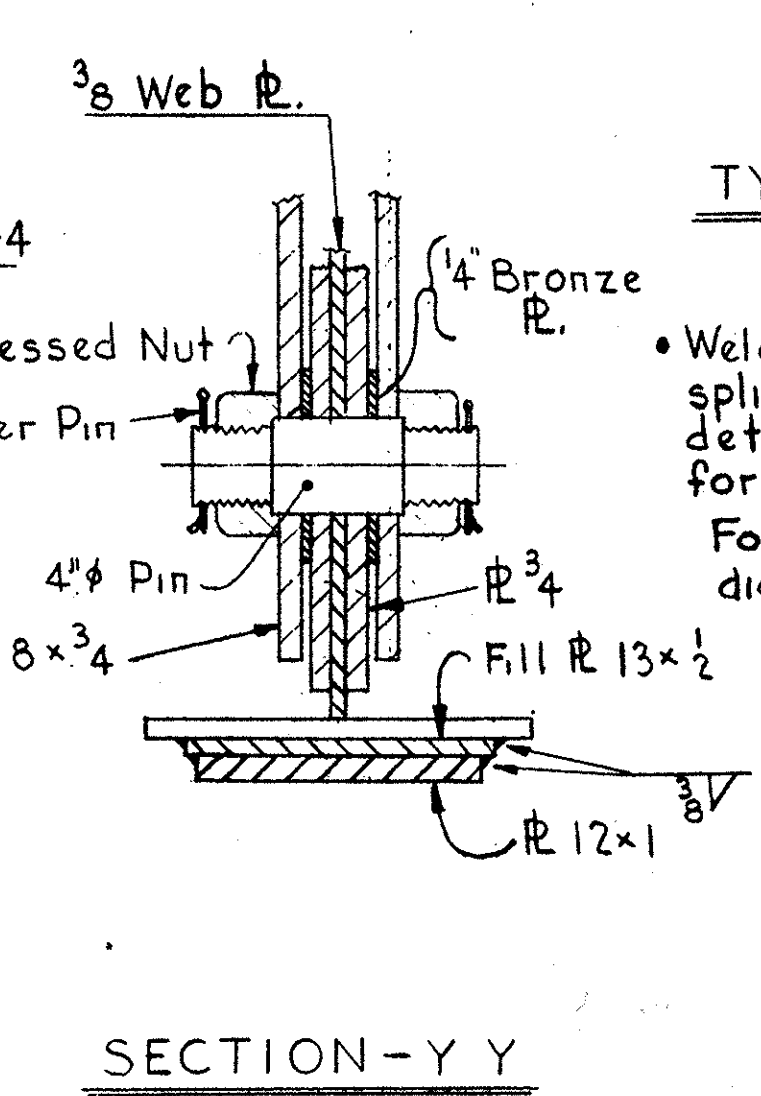
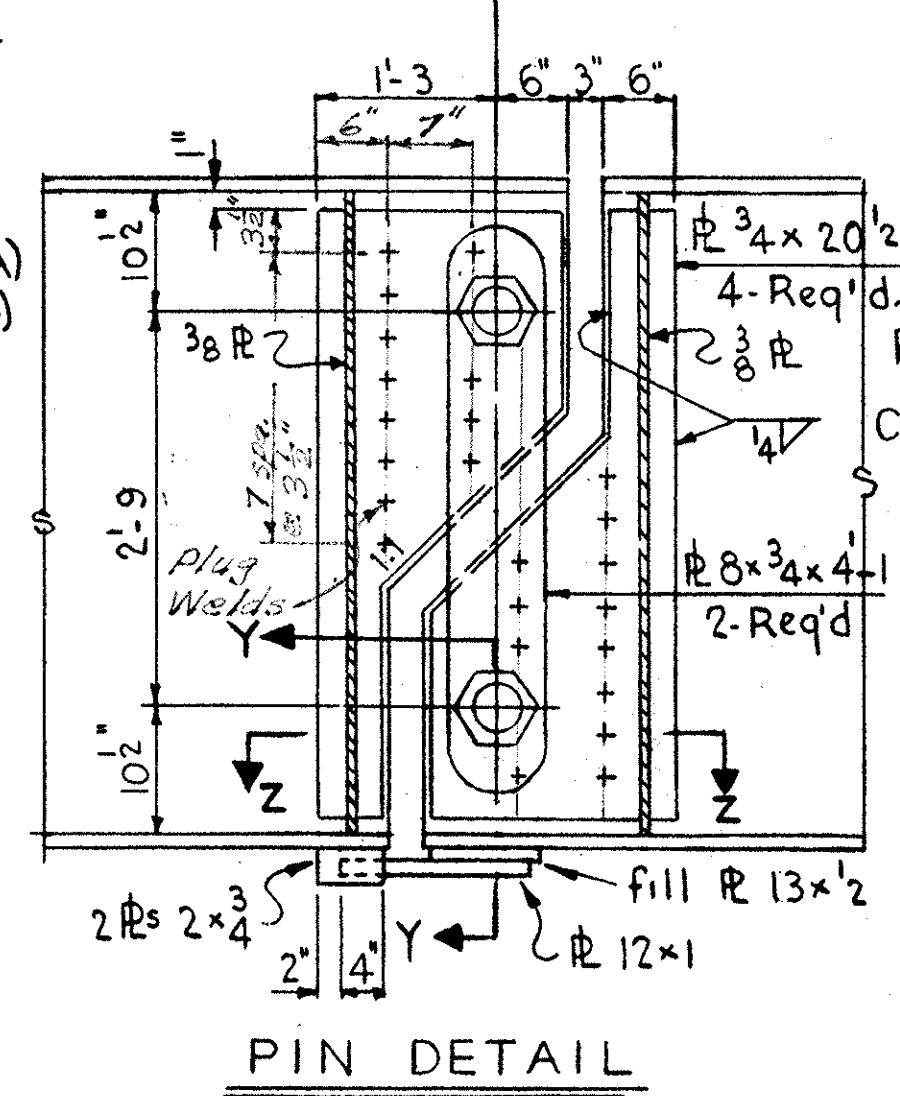
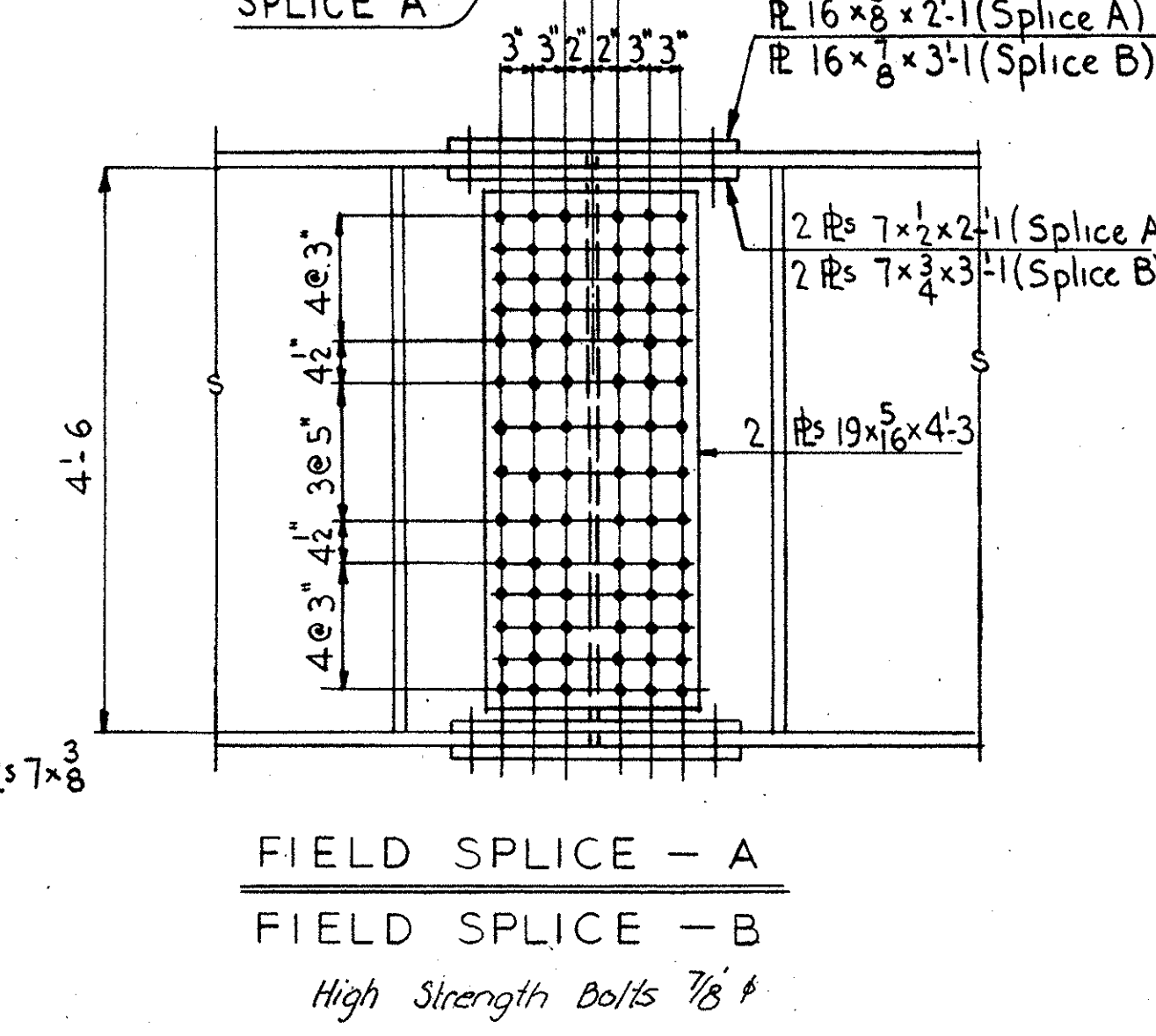
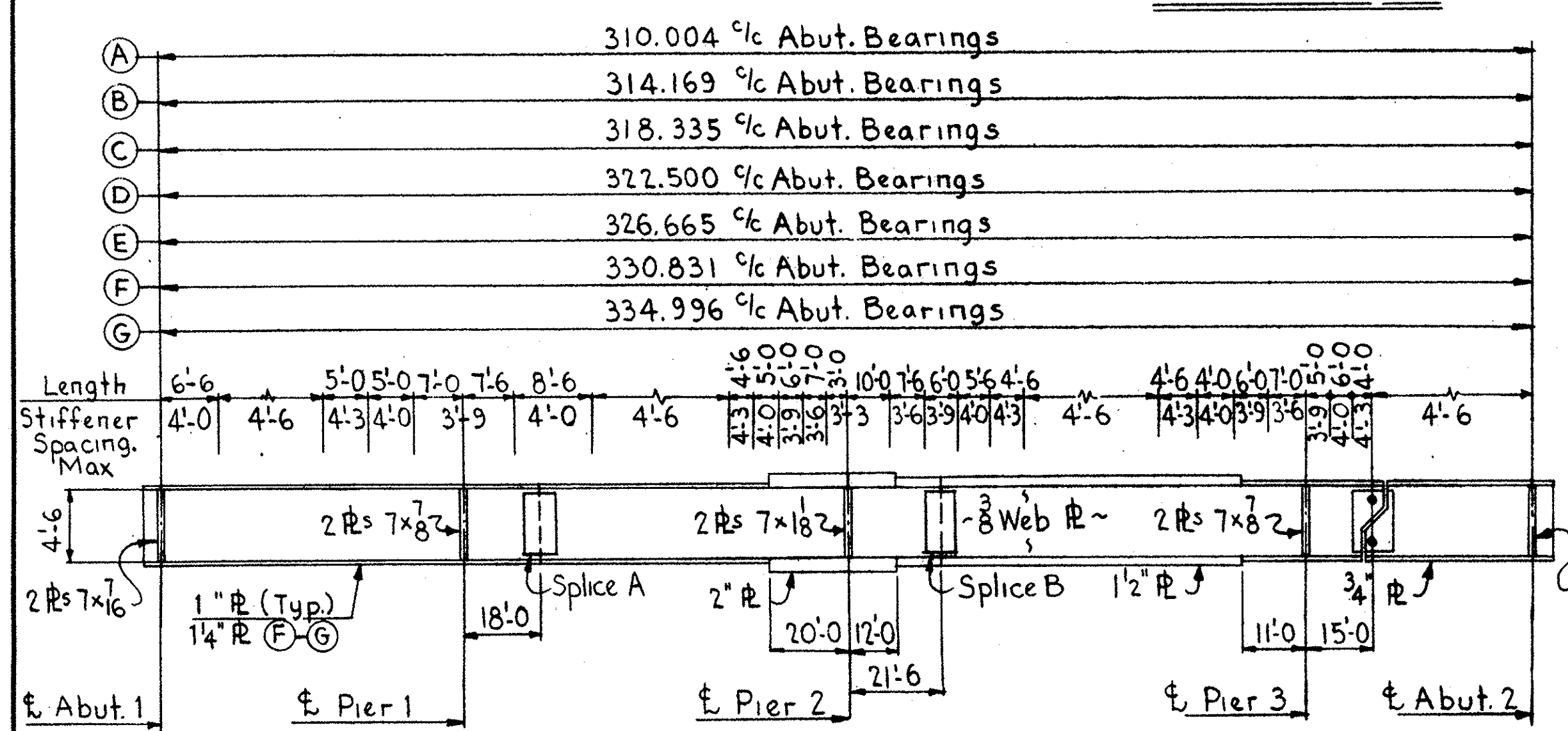
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	bo		LRD	L.G.H. 9-18-63	

MAHONING COUNTY
MAH-18-15.50

Note:
Contractor shall submit to the Director for approval, 3 prints showing proposed erection procedure for the plate girders.



MARK	Thickness	Minimum size of Fillet Weld
A	3/4 to 1	1/4
	1 1/2	5/16
	2	3/8



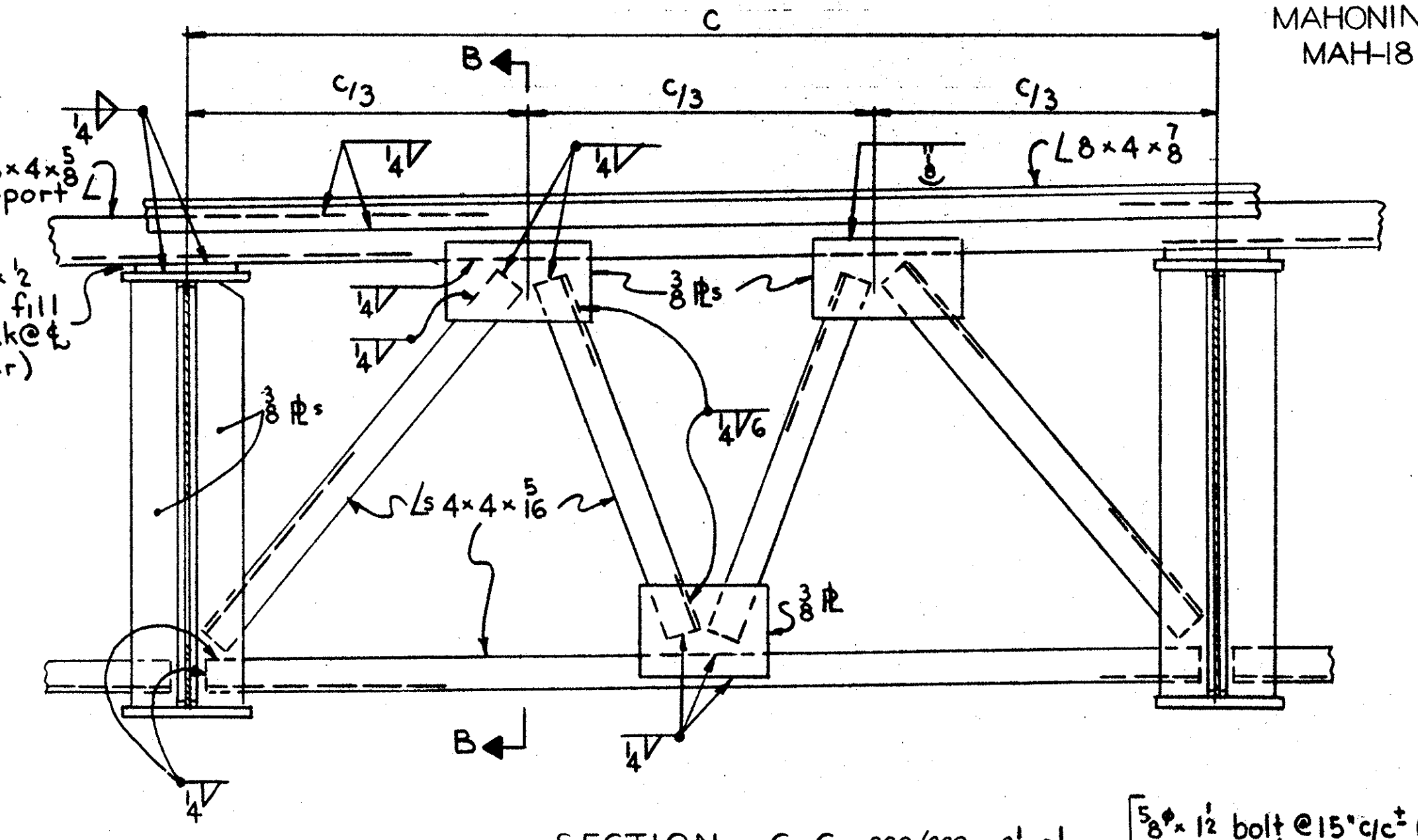
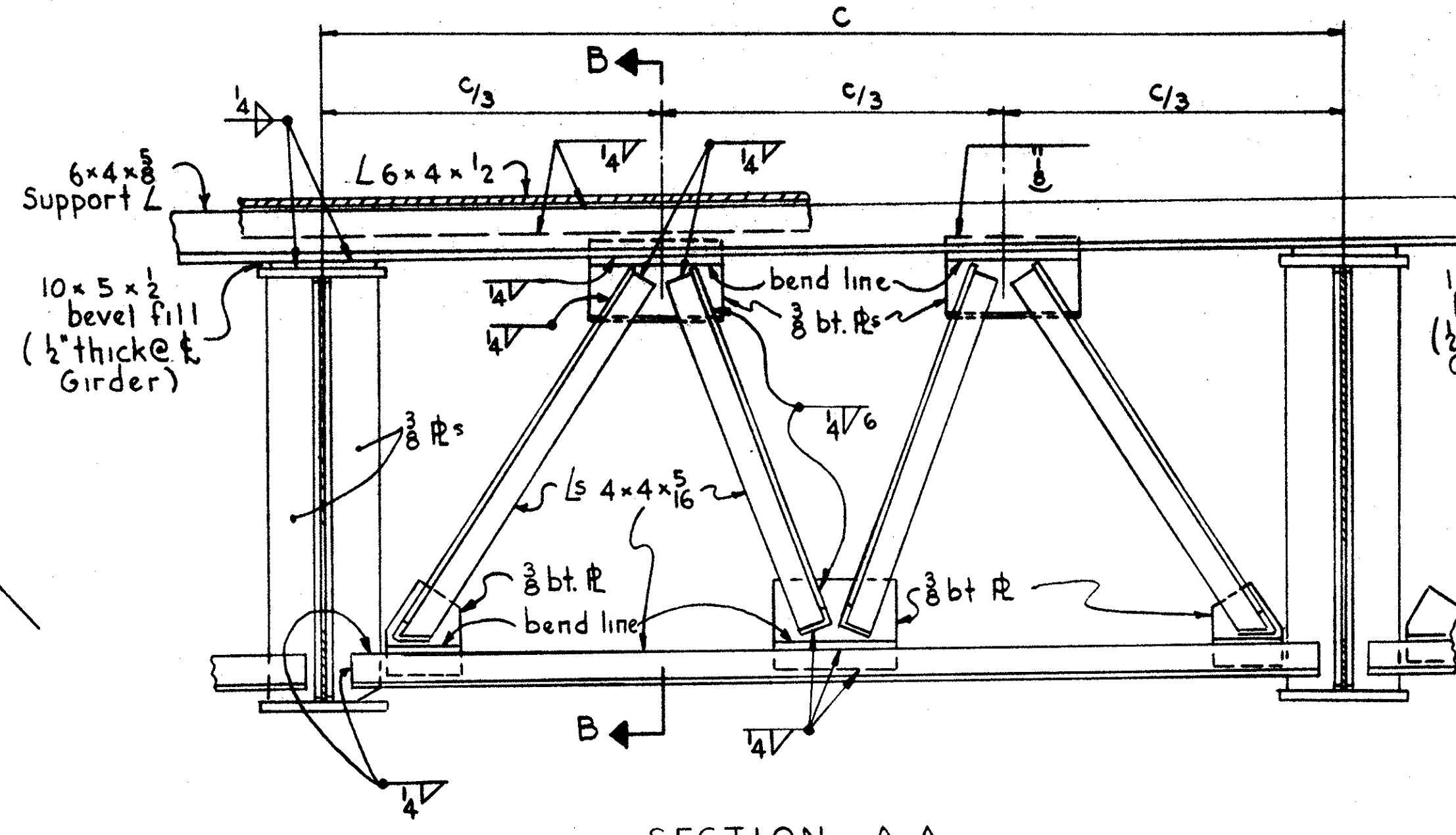
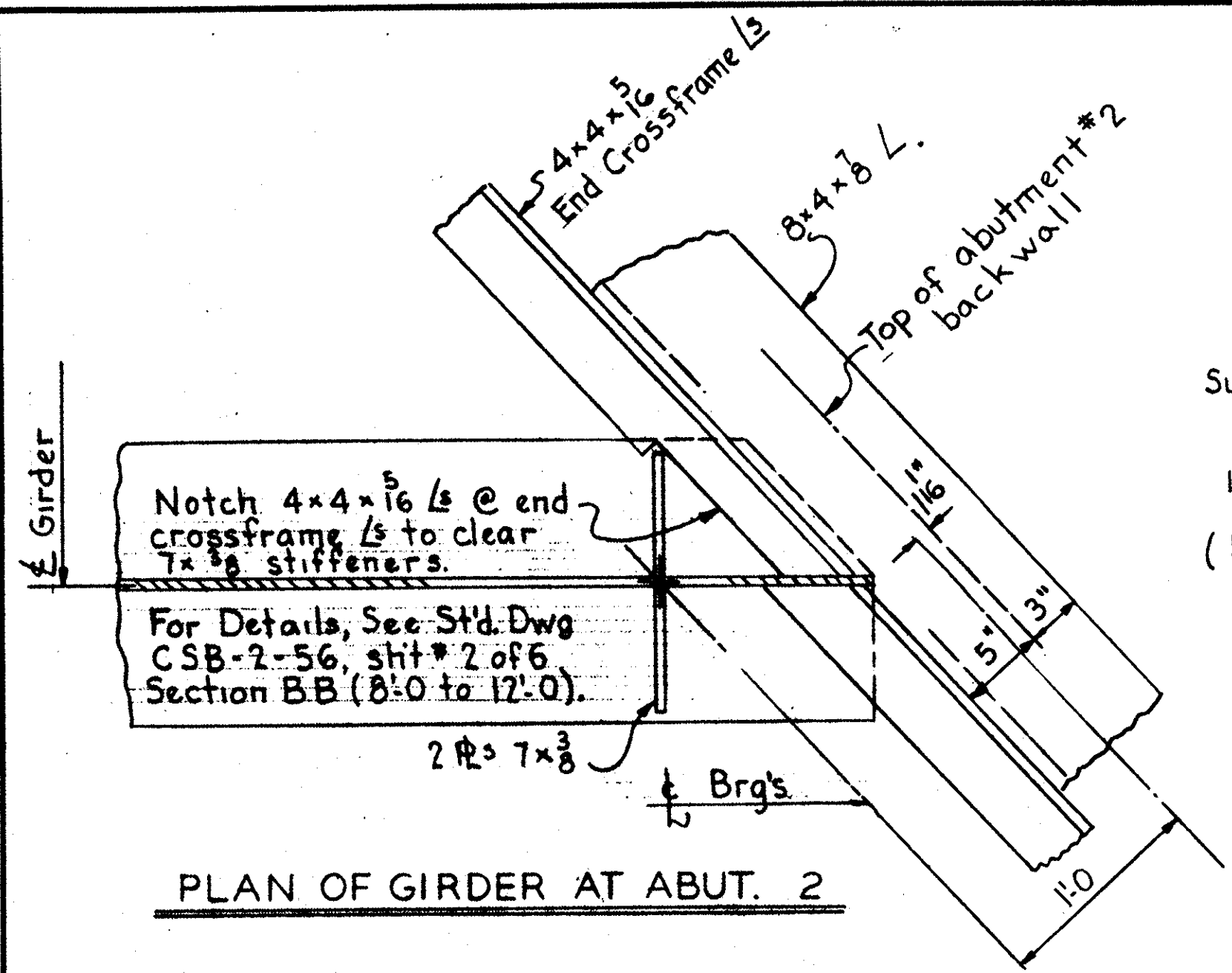
TYPICAL CROSS SECTION
Welded Plate Girders: If additional shop splices are necessary, their locations and detail shall be submitted to the Director for approval.
For deflection curve and camber diagram, See sht #231

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

FRAMING PLAN
BRIDGE NO. MAH-18-1629
UNDER VESTAL ROAD
MAHONING COUNTY STA. 559+80.95

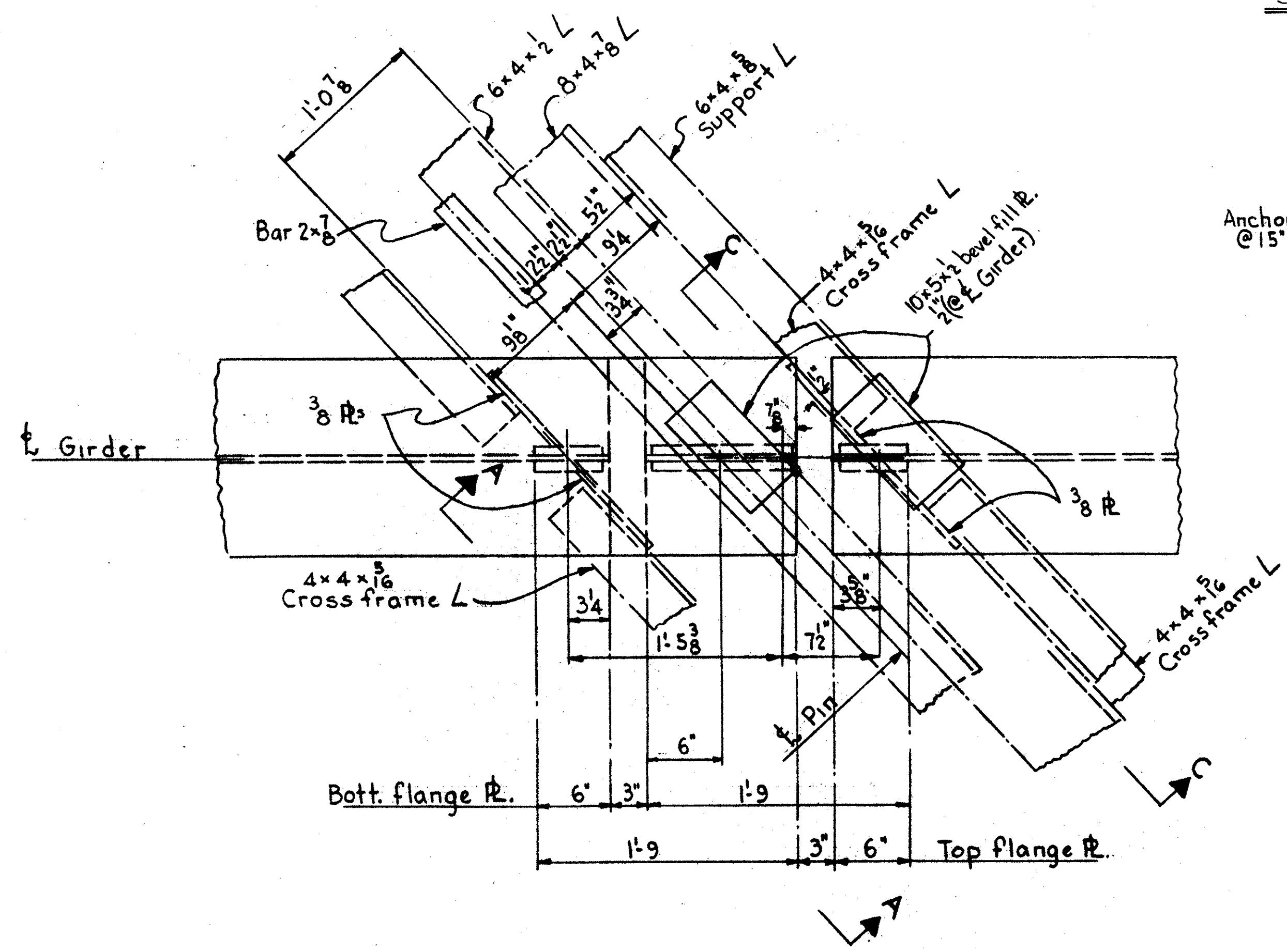
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	bo		LRD	L.G.H. 9-18-63	8-19-65

MAHONING COUNTY
MAH-18-15.50

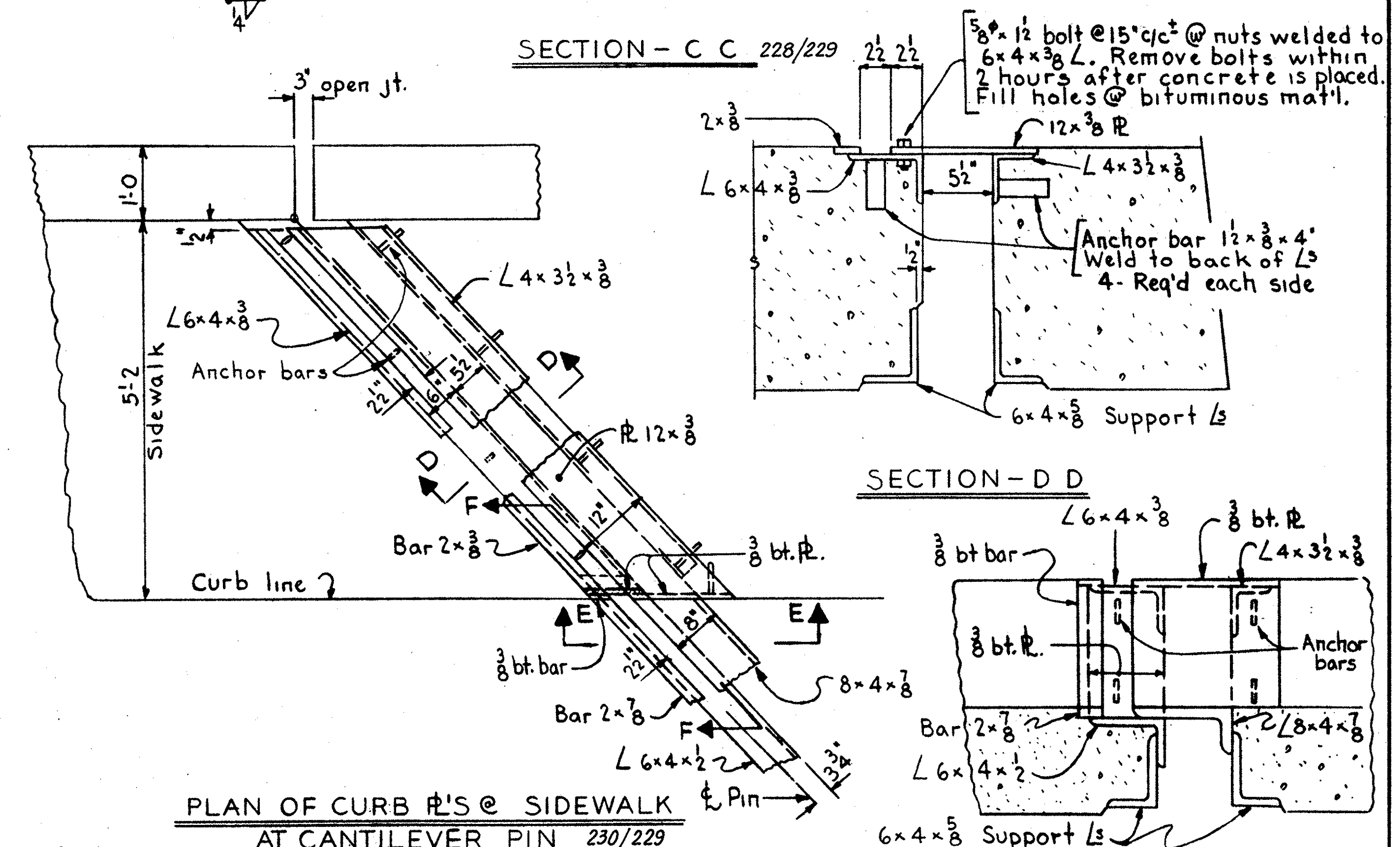
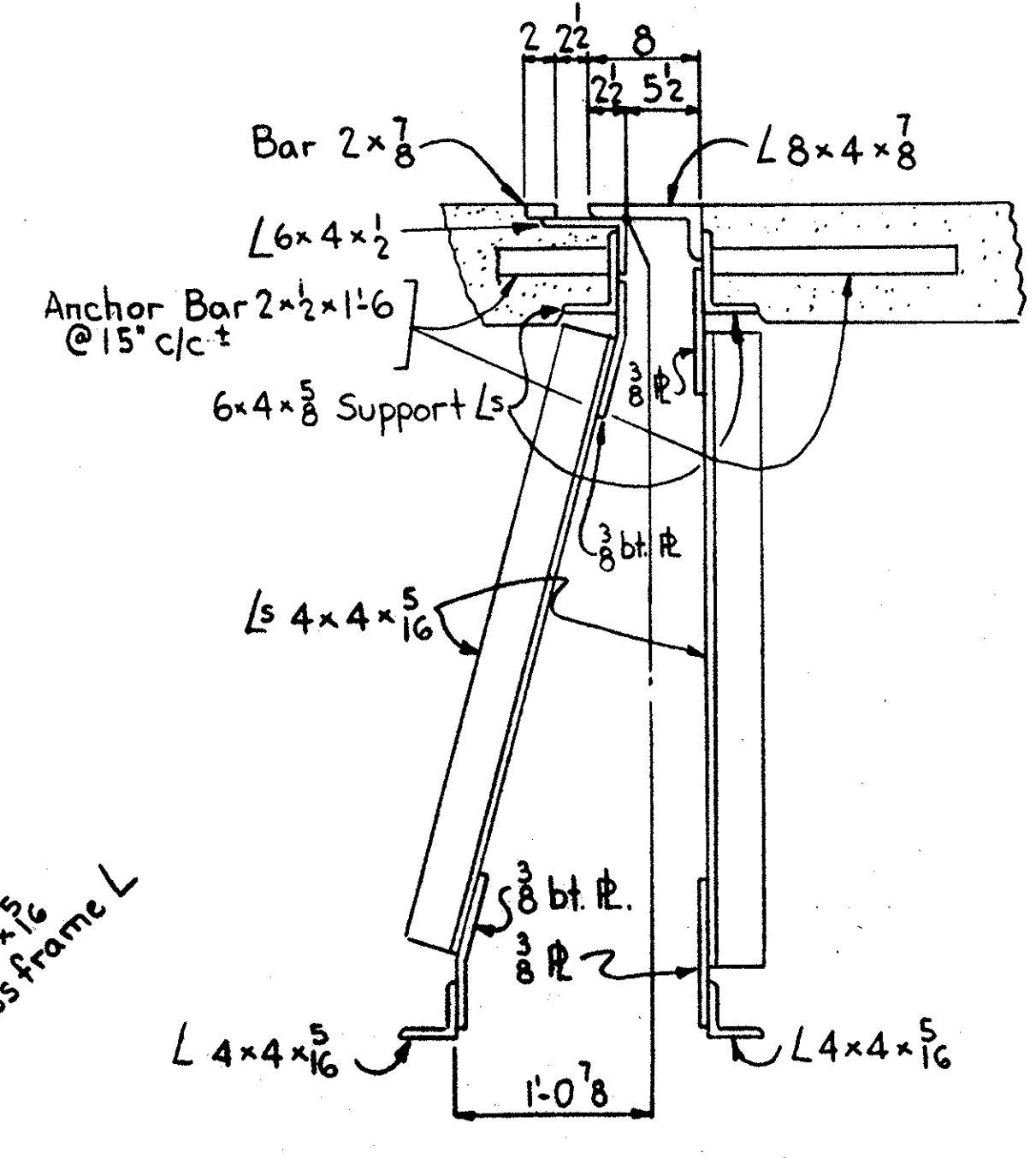


SECTION - A A

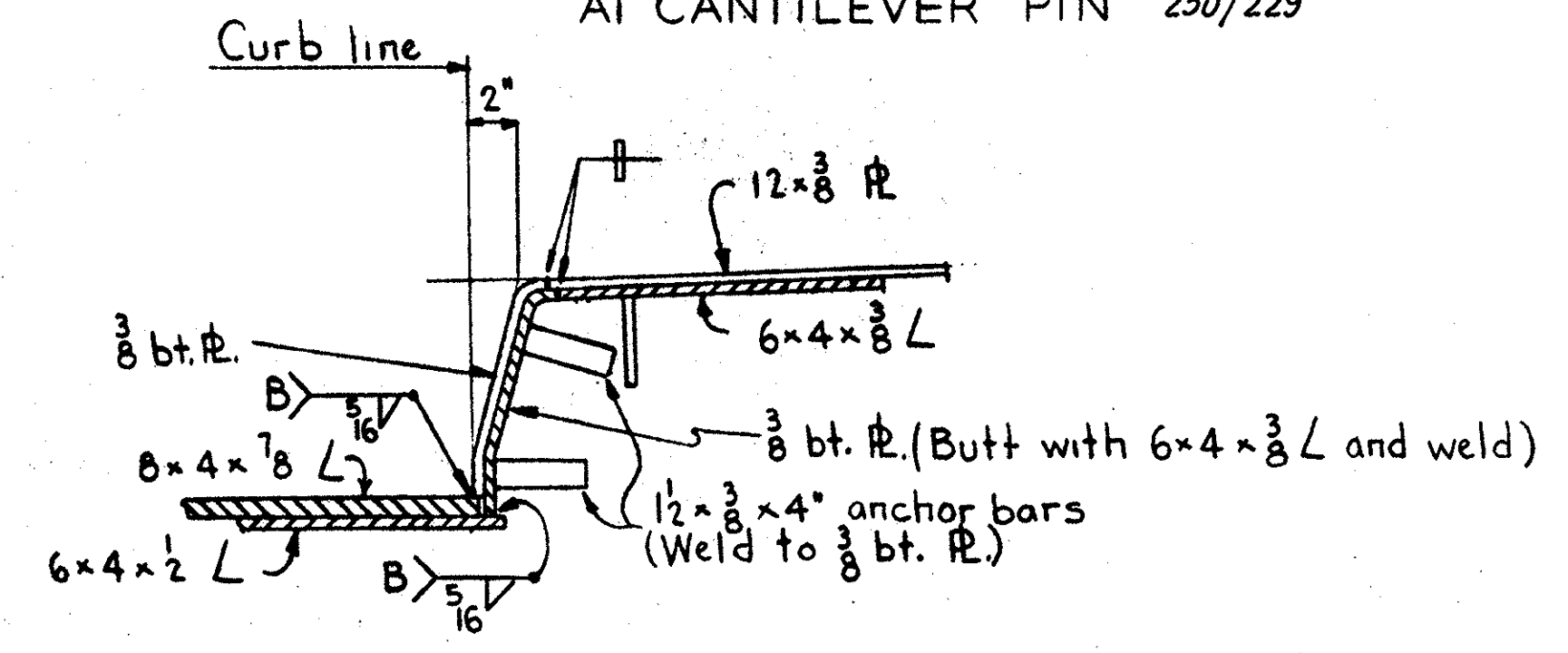
SECTION - C C 228/229



PLAN - X X 228/229



PLAN OF CURB R'S @ SIDEWALK AT CANTILEVER PIN 230/229



MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

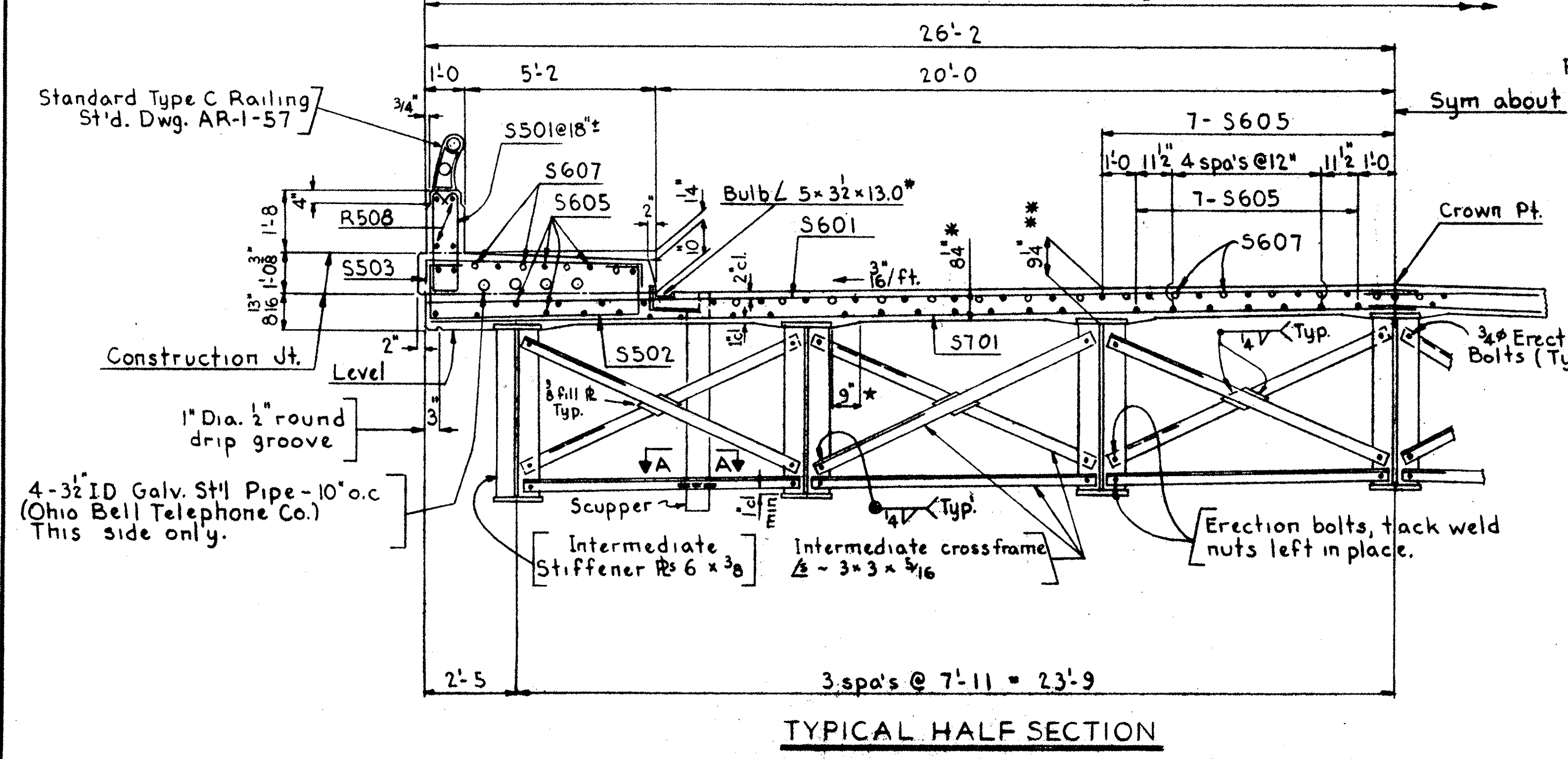
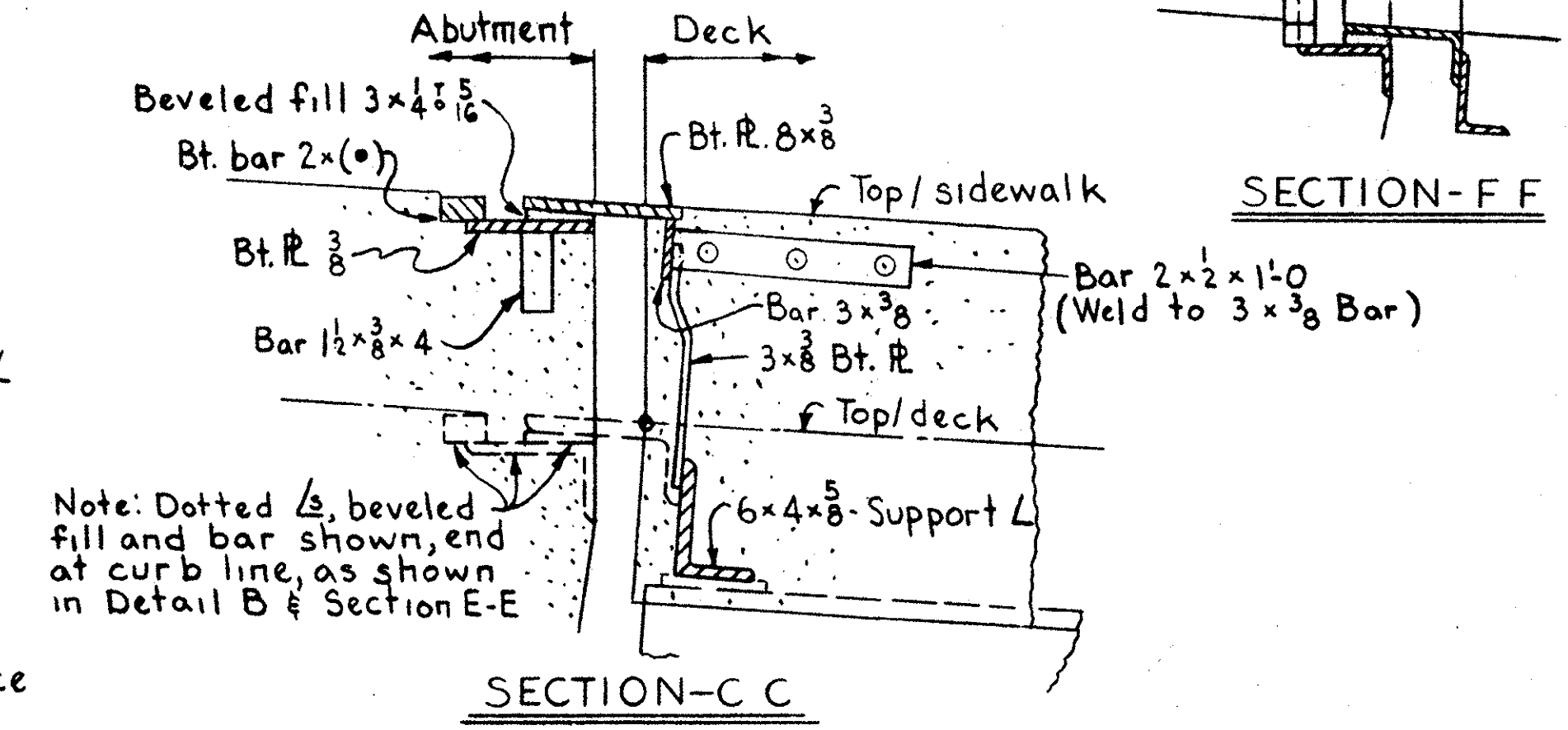
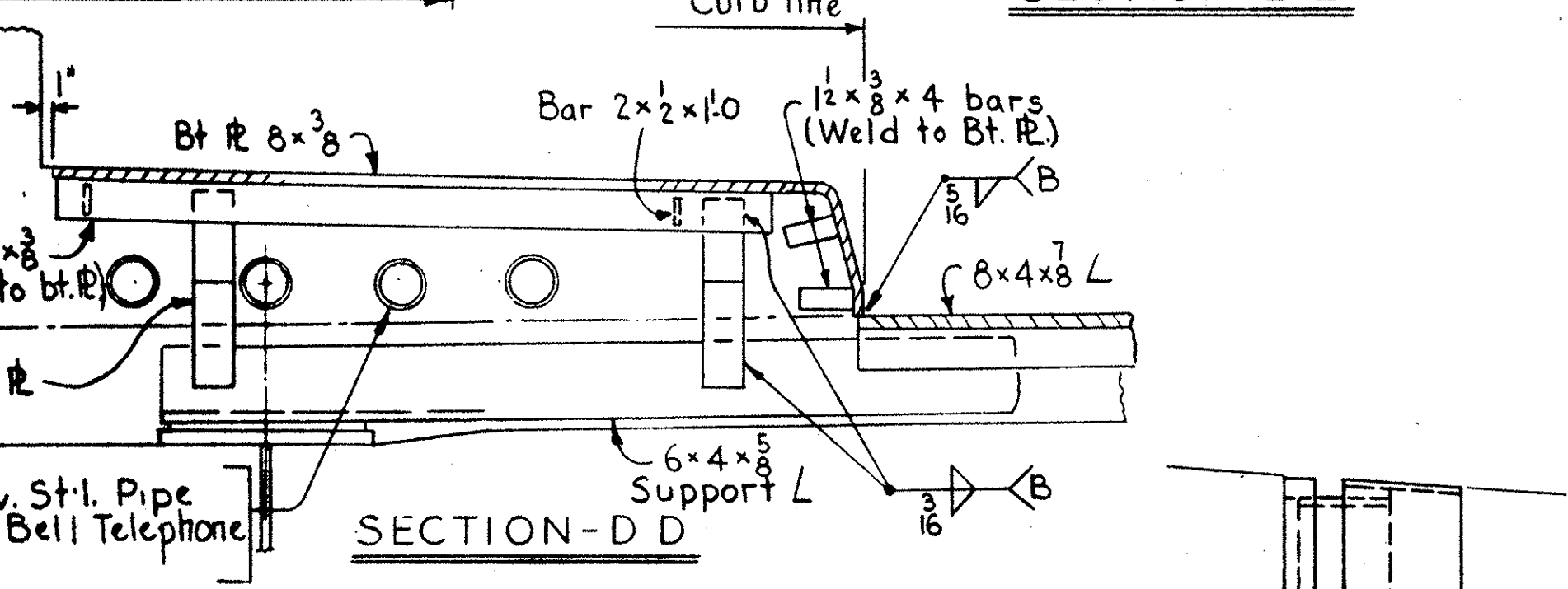
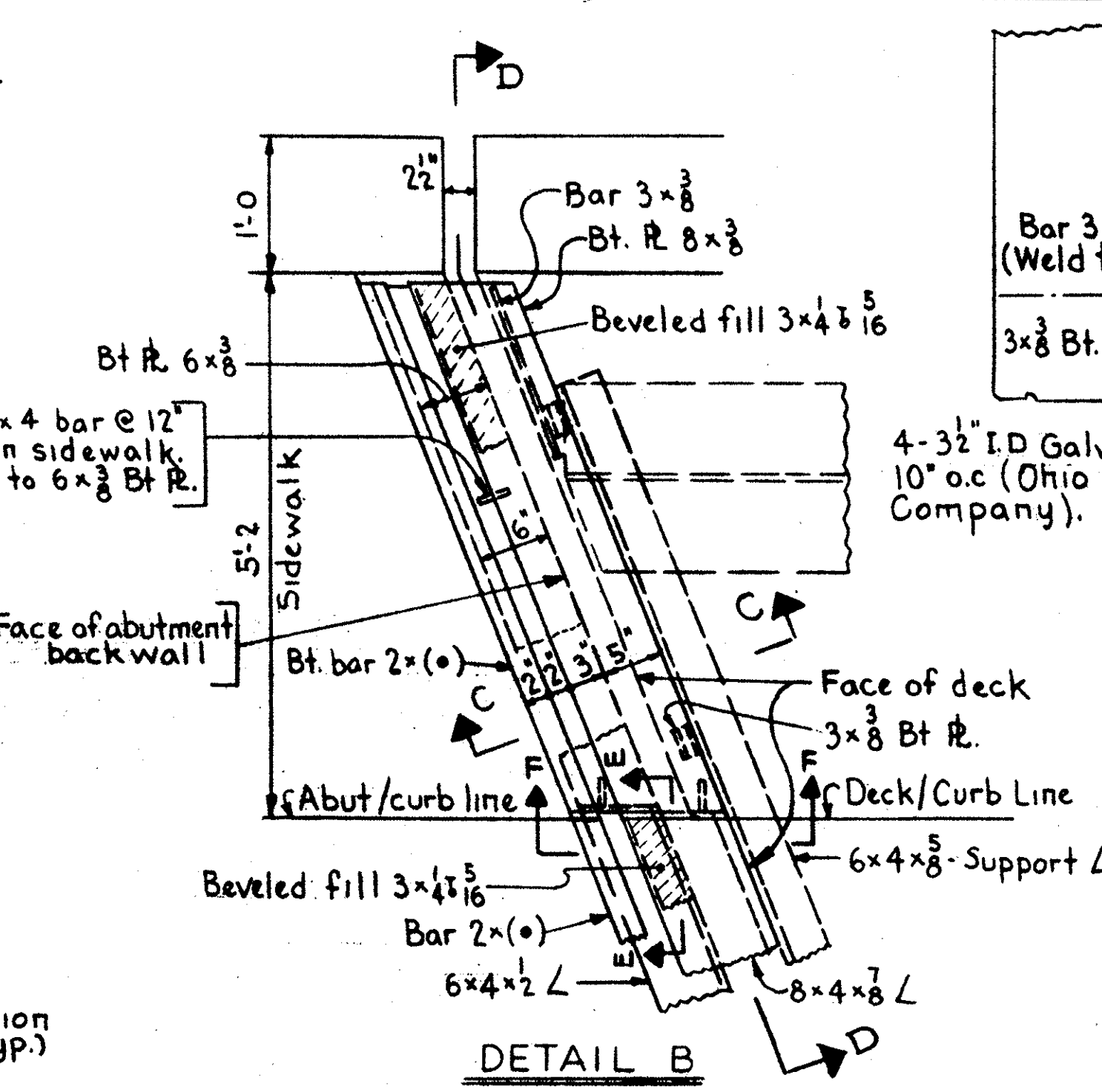
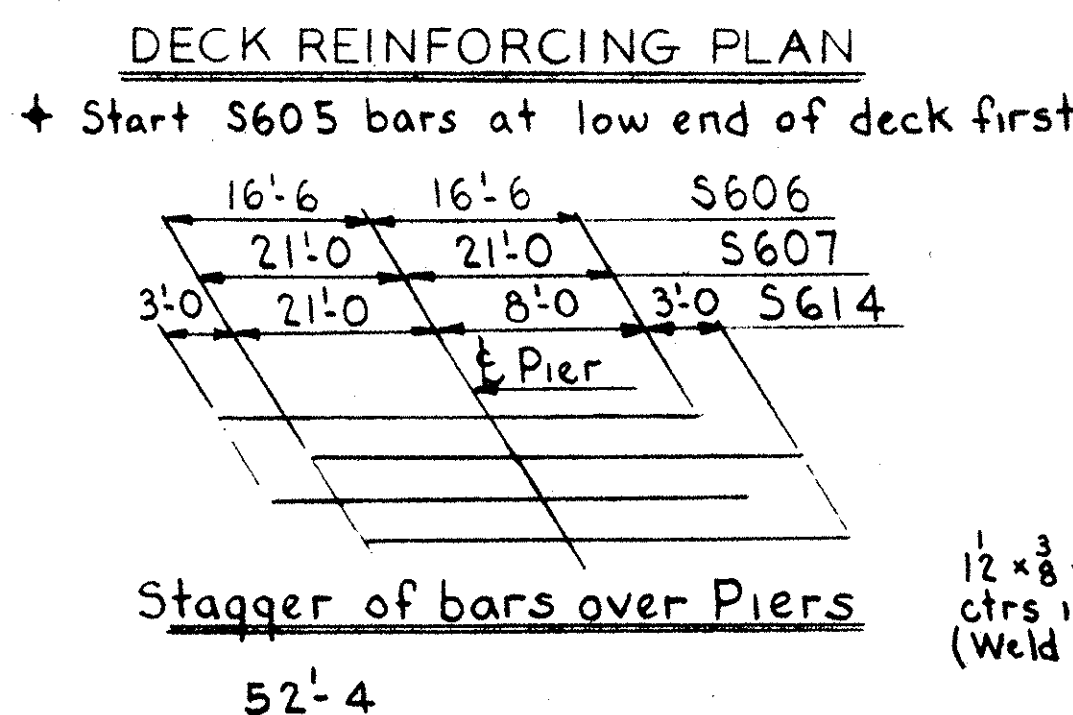
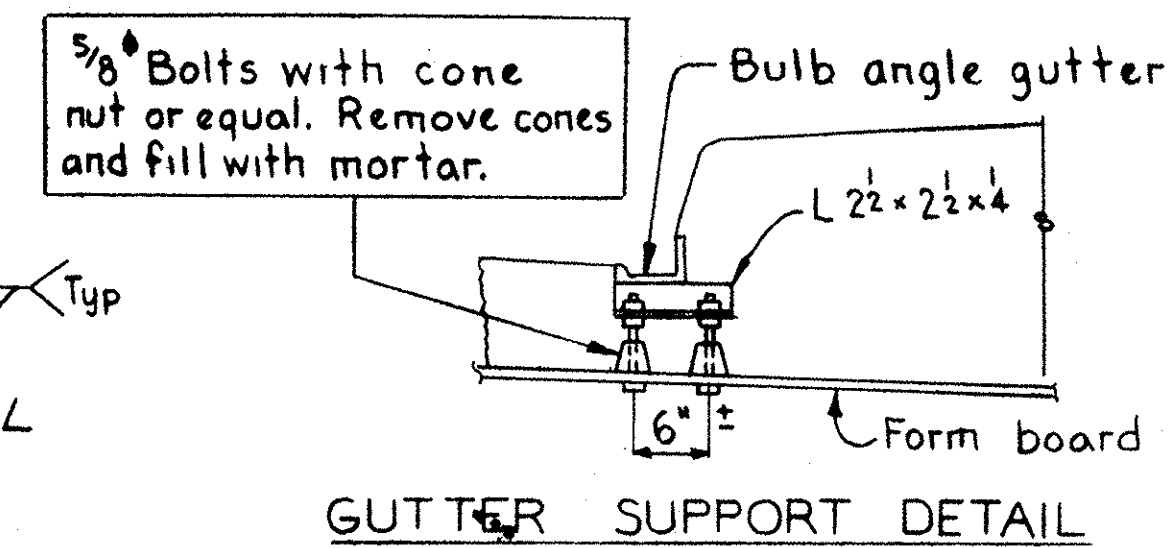
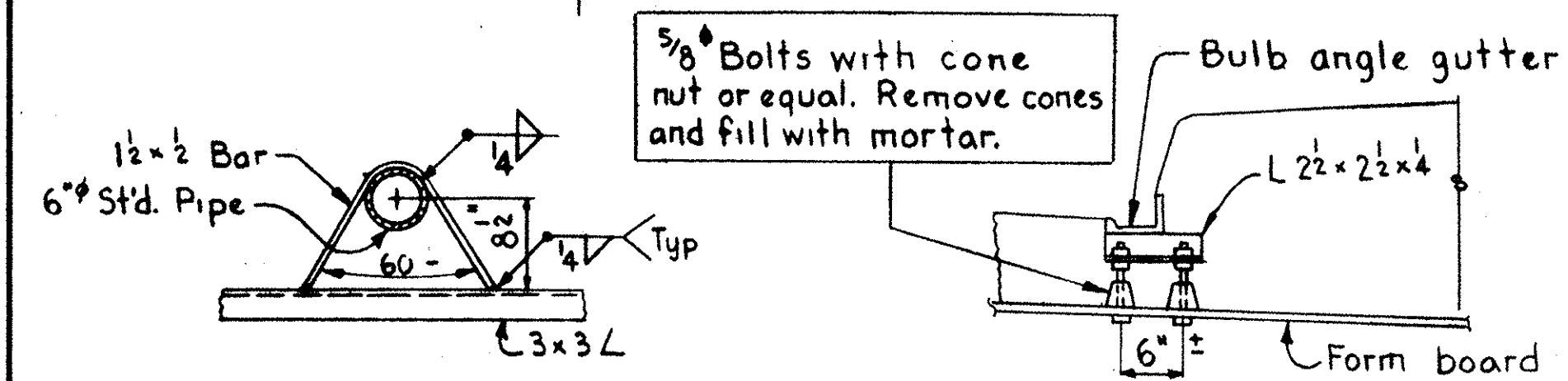
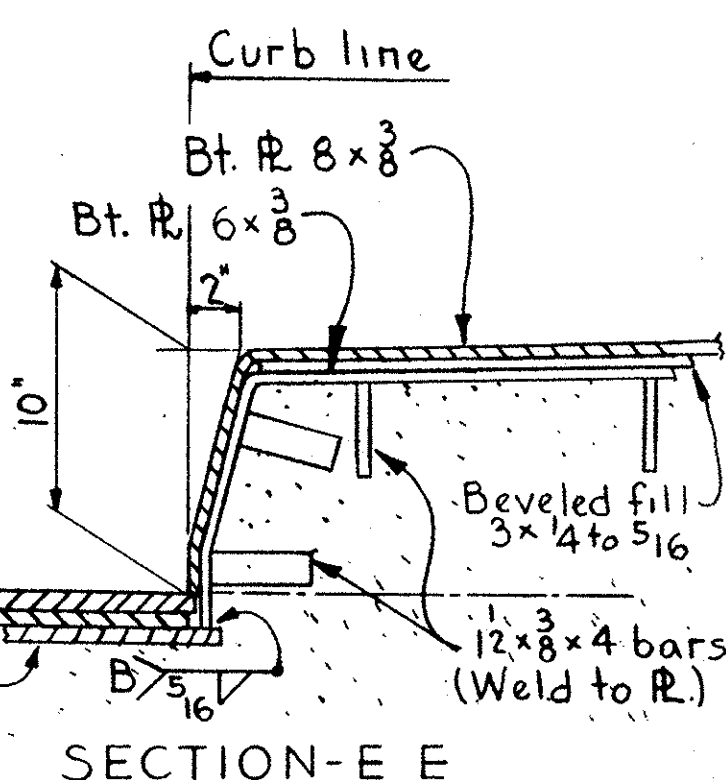
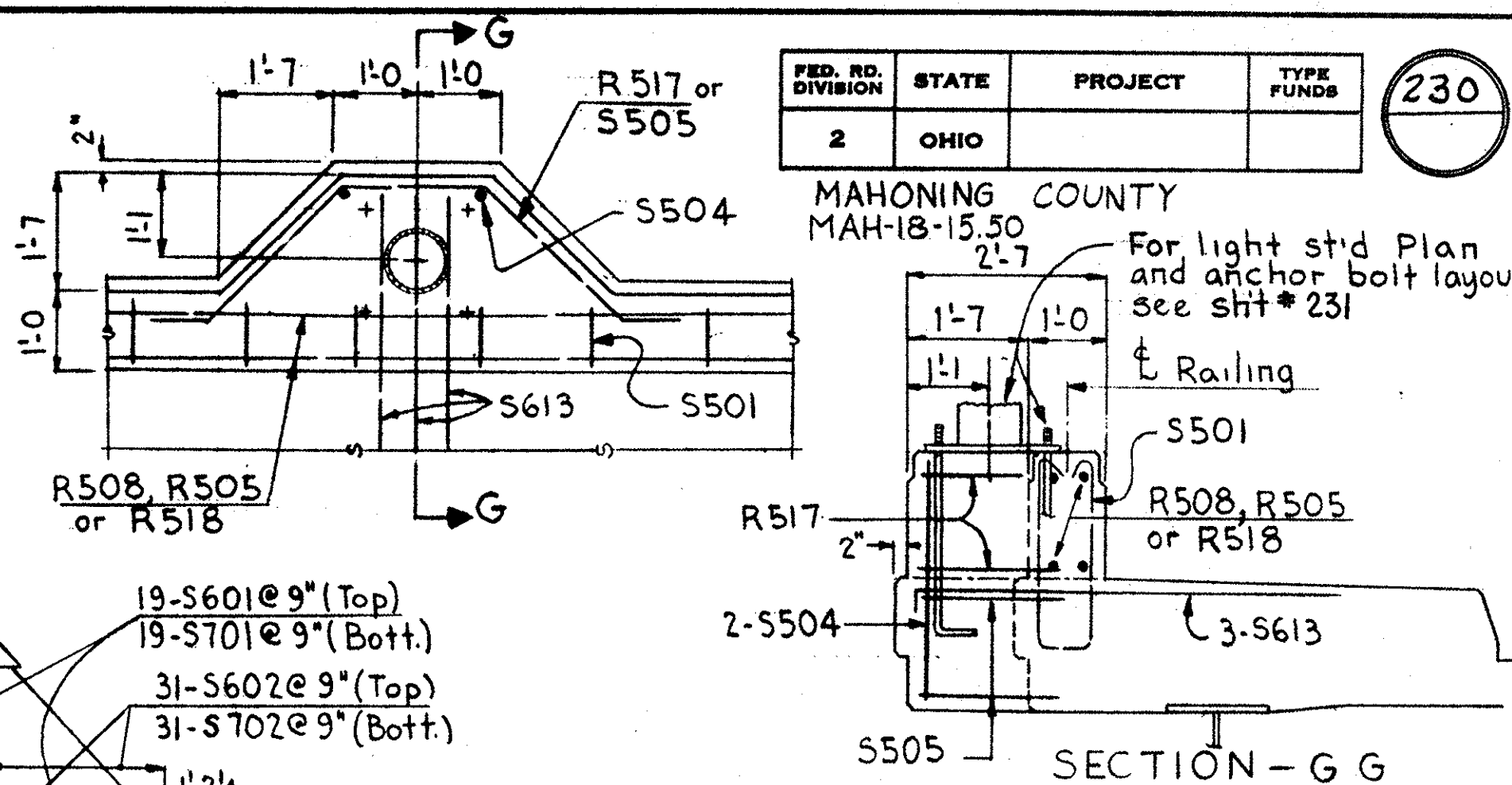
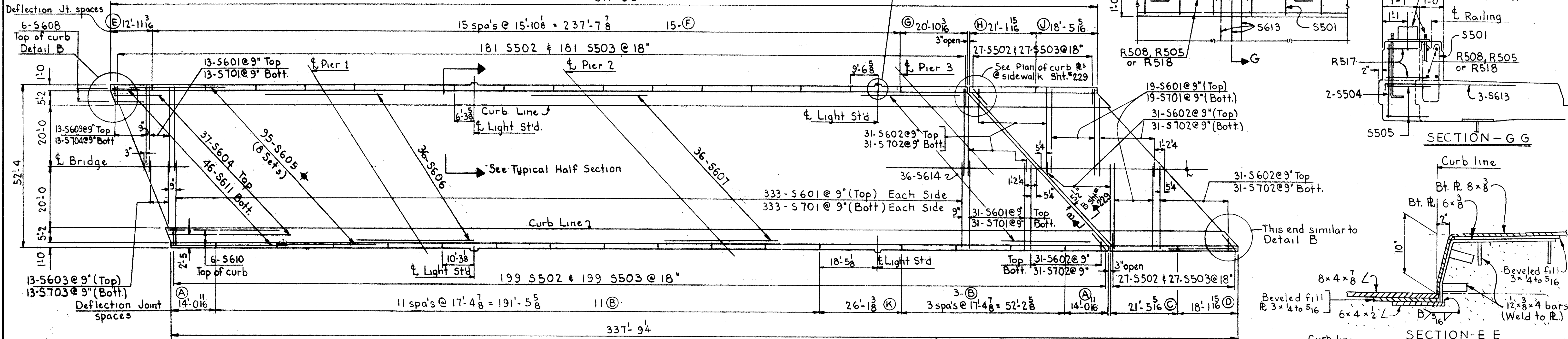
END DAM & CURB DETAILS
AT CANTILEVER PIN
BRIDGE NO MAH-18-1629
UNDER VESTAL ROAD
MAHONING CO. STA 559+80.95

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
S.J.F.	bo		LRD	L.G.H. 9-18-63	

- (A) - 4 R506, 10 S501
- (B) - 4 R505, 12 S501
- (C) - 4 R503, 15 S501
- (D) - 4 R504, 12 S501
- (E) - 4 R507, 9 S501
- (F) - 4 R508, 11 S501
- (G) - 4 R509, 14 S501
- (H) - 4 R510, 15 S501
- (J) - 4 R511, 13 S501
- (K) - 4 R518, 18 S501

FED. RD. DIVISION	STATE	PROJECT	TYPE FUNDS	230
2	OHIO			

MAHONING COUNTY
MAH-18-15.50

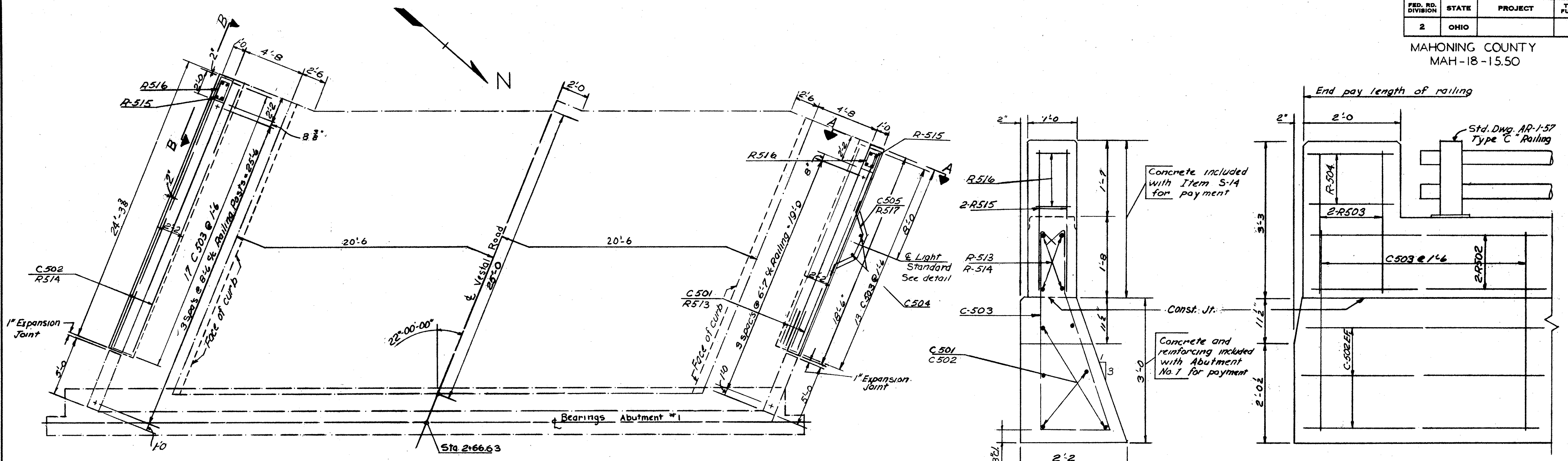


- * Slab thickness shown includes 1" monolithic wearing surface
- * Deck Slab Haunch: The haunch in the deck slab adjacent to the top of steel girders, which is shown as 9" wide may vary from this dimension between the limits of 6" and 12", except that the maximum slope shall not exceed 3" per ft. Payment for deck slab concrete shall be based on 9" width.
- ** This is the nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the girder may not have the exact camber or conformation required to place it parallel to the finished grade.

(*) For bar size see abutment No 1 & No 2 backwall sketches
Refer to Std Dwg CSB-2-56 sheet 3 of 6 for details of bulb angle gutter and Scupper

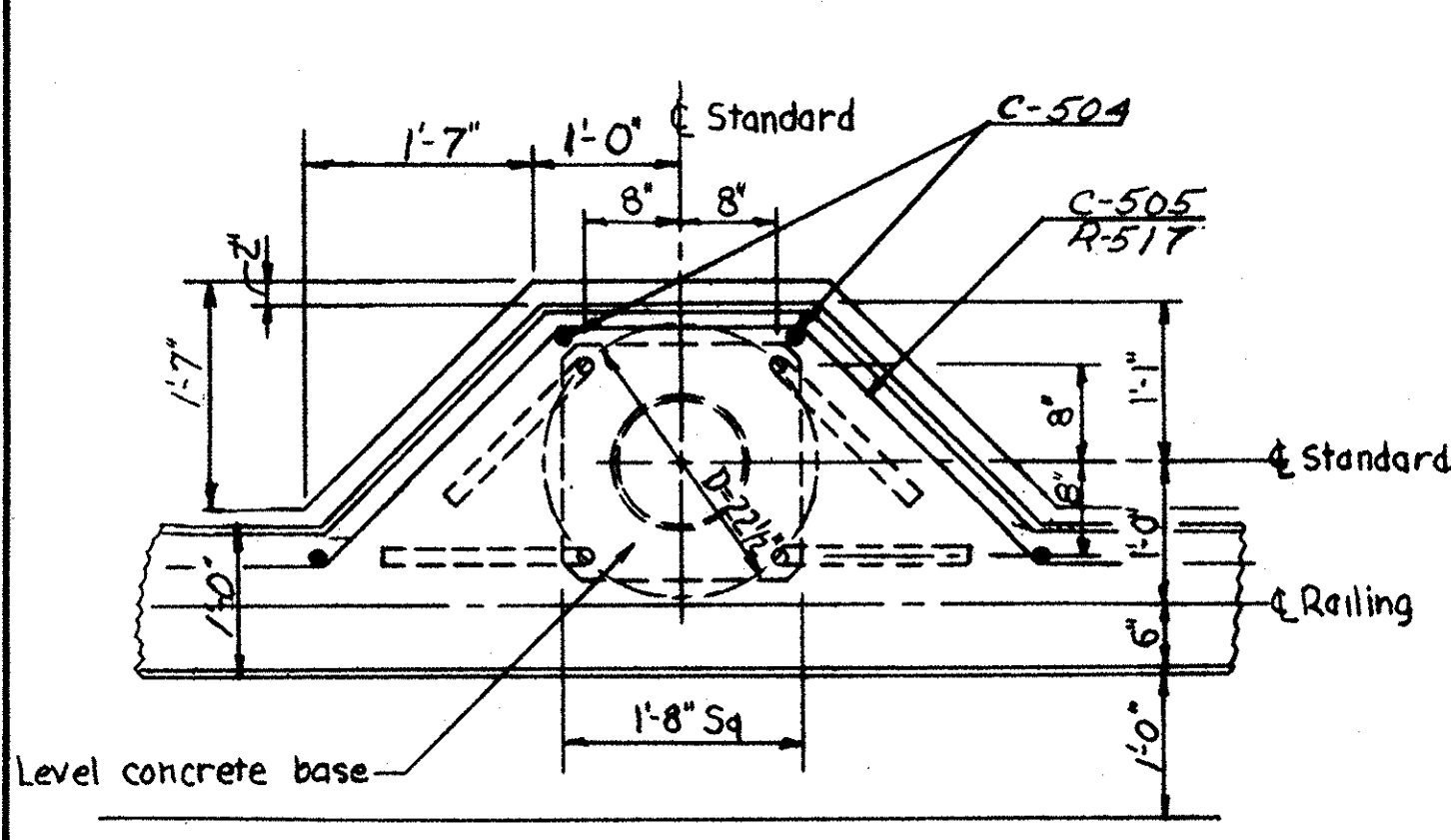
MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
DECK REINFORCING PLAN BRIDGE NO. MAH-18-1629 UNDER VESTAL ROAD					
MAHONING COUNTY STA. 559+80.95					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	bo		LRD	L.G.H. 9-18-63	

MAHONING COUNTY
MAH-18-15.50

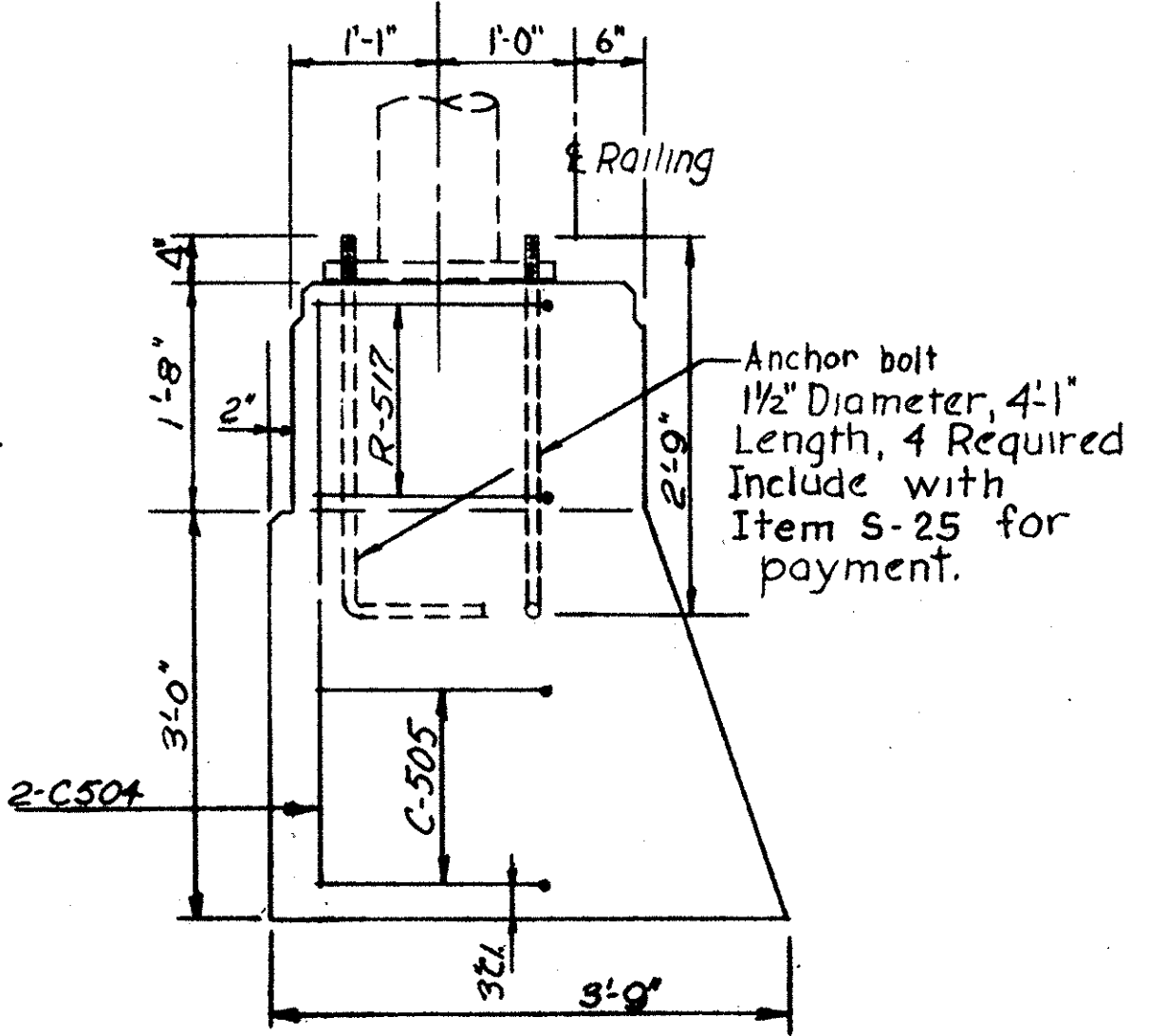


RAILING PLAN AT ABUTMENT #1

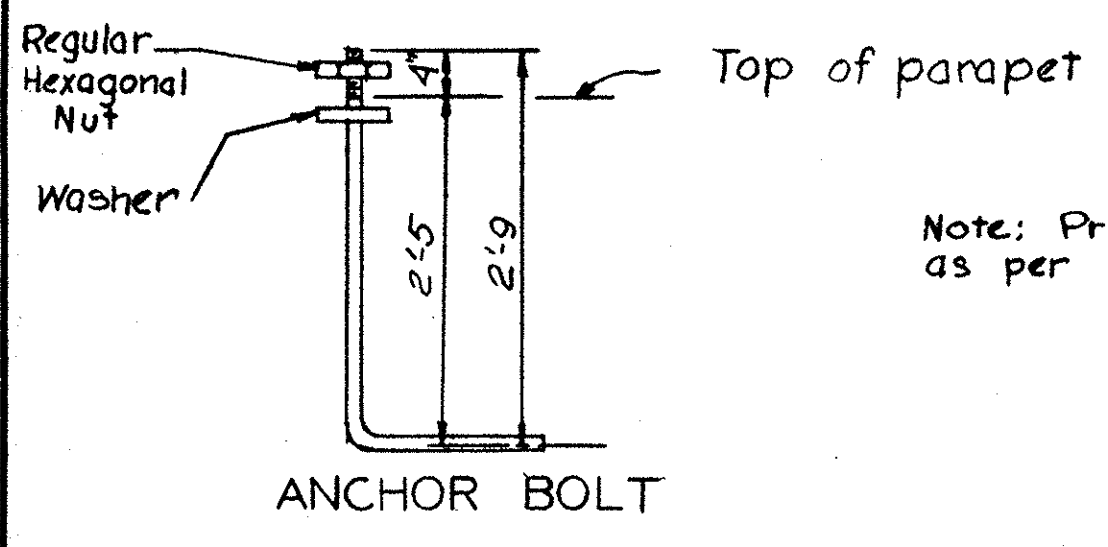
Note: Concrete shall be Class "E" up to construction joint.



LIGHT STANDARD PLAN



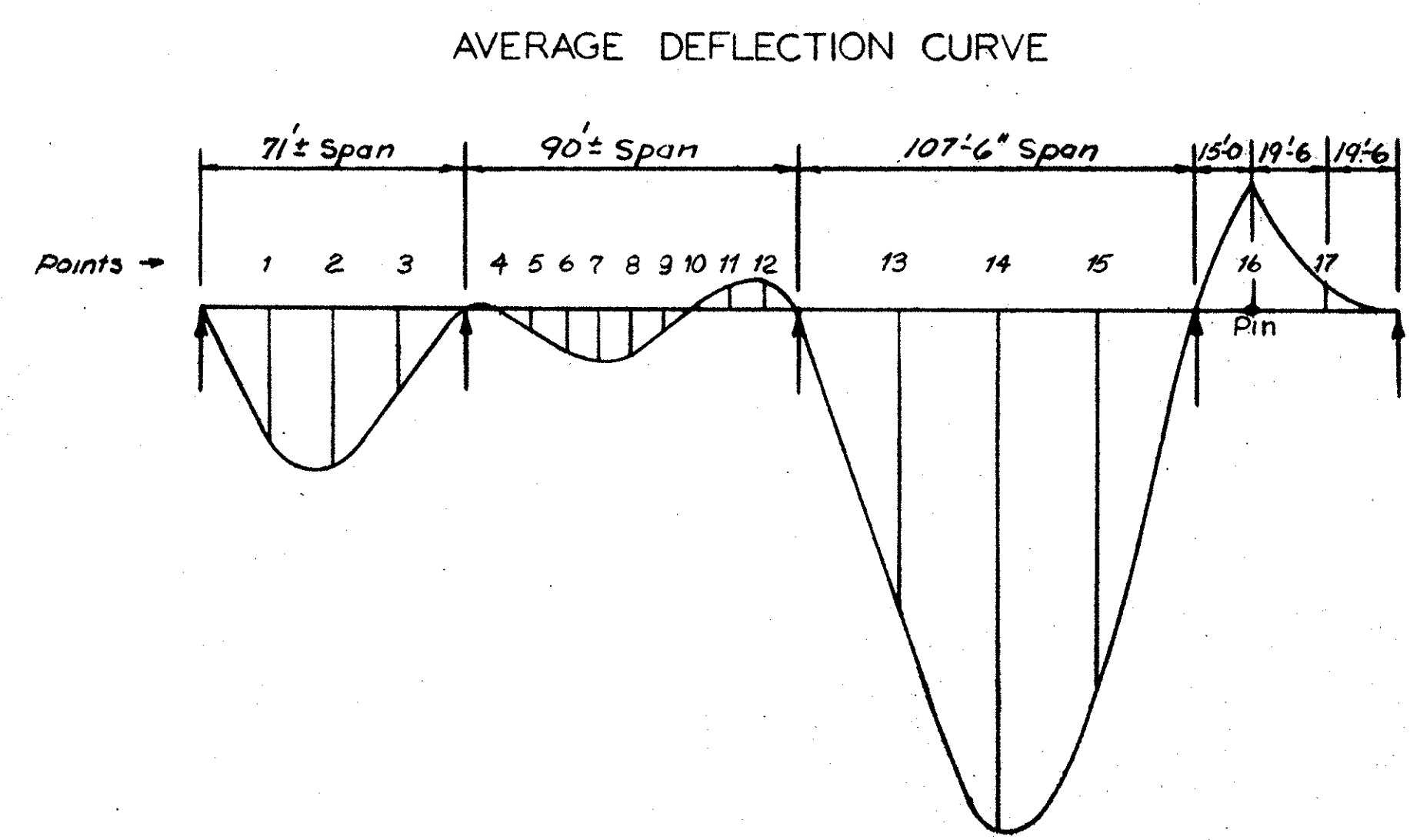
LIGHT STANDARD ELEVATION



Note: Provide electrical ground as per Item S-25.10.

DEFLECTION & CAMBER																																																	
Point	1		2		3		4		5		6		7		8		9		10		11		12		13		14		15		16		17																
Beam No. & Avg.	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	A	Avg	G	-															
Deflection due to weight of steel	.05	.06	.07	.06	.07	.08	.03	.04	.04	.00	.00	.00	.01	.02	.00	.02	.04	.00	.02	.05	.01	.02	.05	.02	.01	.03	.03	.00	.02	.03	.01	.02	.02	.01	.17	.16	.15	.30	.28	.27	.22	.22	.21	.11	.10	.10	.03		
Deflection due to remaining dead load	.17	.19	.20	.22	.26	.29	.12	.13	.15	.01	.00	.02	.00	.04	.07	.01	.07	.13	.01	.10	.17	.00	.09	.17	.03	.05	.13	.06	.00	.07	.07	.03	.01	.06	.04	.02	.48	.45	.43	.84	.81	.77	.42	.59	.57	.25	.24	.22	.06
Total deflection	.22	.25	.27	.28	.33	.37	.15	.17	.19	.01	.00	.02	.00	.05	.09	.01	.09	.17	.01	.12	.22	.01	.11	.22	.05	.06	.16	.09	.00	.09	.10	.04	.02	.08	.06	.03	.65	.61	.58	1.14	1.09	1.04	.84	.81	.78	.36	.34	.32	.09
Required camber	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		

Average deflection for interior beams is 0.07" less than average shown for exterior beams.
Beam A is on short side - Beam G on long side



MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

RAILING EXTENSION AT ABUT #1
DEFLECTION & CAMBER DIAGRAM
BRIDGE NO. MAH-18-1629
UNDER VESTAL ROAD

MAHONING CO. STA. 559+80.95

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S.J.F.	L.R.D.		D.W.P.	L.G.H. 9-18-63	

REINFORCING STEEL LIST

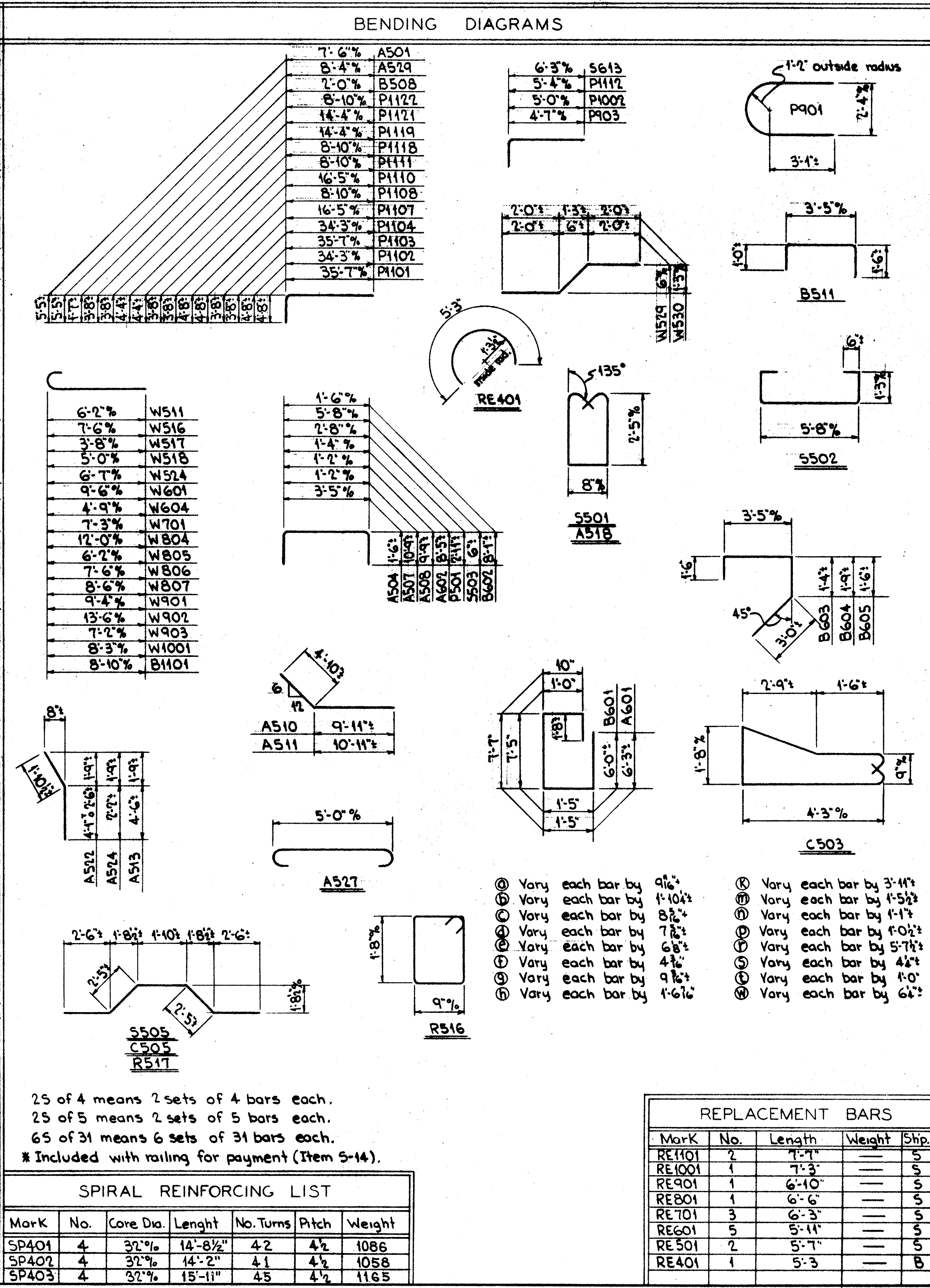
Mark	No.	Length	Weight	Shp.
SUPERSTRUCTURE				
5701	748	27'-2"	41,535	S
5702	65	26'-4" x 2'-1"	5,402	S
5703	13	25'-5" x 3'-3"	381	S
5704	13	26'-5" x 4'-1"	405	S

Mark	No.	Length	Weight	Shp.
RAILING EXTENSION				
5601	748	27'-0"	30,334	S
5602	65	26'-4" x 2'-1"	3,969	S
5603	13	25'-3" x 3'-1"	277	S
5604	37	10'-0" x 35'-0"	1,250	S
5605	760	34'-10"	39,763	S
5606	36	36'-0"	1,947	S
5607	36	45'-0"	2,433	S
5608	6	9'-0"	81	S
5609	13	26'-5" x 4'-1"	298	S
5610	6	35'-0"	315	S
5611	46	8'-6" x 36'-0"	1,537	S
5612	95	38'-4"	5,470	S
5613	12	6'-11"	125	B
5614	36	32'-0"	1,730	S

Mark	No.	Length	Weight	Shp.
WINGWALLS 'C' & 'D'				
W1001	15	9'-8"	624	B
W901	42	10'-7"	1,511	B
W902	15	14'-9"	752	B
W903	14	8'-5"	401	B
W801	20	8'-11"	476	S
W802	21	18'-10" x 2'-9"	1,343	S
W803	1	29'-1"	78	S
W804	22	13'-1"	769	B
W805	21	7'-3"	407	B
W806	43	8'-7"	985	B
W807	64	9'-7"	1,638	B
W701	29	8'-1"	479	B
W702	31	7'-9"	441	S
W703	32	27'-6" x 16'-8"	1,444	S
W704	1	27'-6"	56	S
W601	16	10'-2"	244	B
W602	14	6'-0"	126	S
W603	15	10'-1" x 20'-10"	348	S
W604	16	5'-5"	130	B
W501	8	9'-10" x 20'-7"	127	S
W502	32	20'-8"	640	S
W503	25	16'-9" x 5'-0"	91	S
W504	2	23'-3"	48	S
W505	8	18'-7" x 28'-10"	198	S
W506	2	22'-2"	46	S
W507	25	18'-0" x 6'-2"	101	S
W508	5	16'-3" x 20'-7"	96	S
W509	5	23'-4" x 19'-2"	111	S
W510	1	19'-6"	20	S
W511	42	6'-9"	296	B
W512	4	23'-0" x 19'-10"	89	S
W513	4	17'-11" x 21'-1"	81	S
W514	1	22'-2"	23	S
W515	32	31'-8"	1057	S
W516	17	8'-1"	143	B
W517	16	4'-3"	71	B
W518	33	5'-7"	192	B
W519	6	34'-1"	213	S
W520	11	16'-5" x 26'-11"	249	S
W521	1	27'-3"	28	S
W522	3	32'-8"	102	S
W523	25	27'-1" x 10'-3"	156	S
W524	32	7'-2"	239	B
W525	32	5'-8" x 16'-8"	373	S
W526	12	5'-5" x 16'-5"	131	S

Mark	No.	Length	Weight	Shp.
REAR ABUTMENT				
B101	130	10'-5"	7,195	B
B102	63	8'-5"	2,817	S
B103	42	15'-9"	3,515	S
B104	12	21'-9"	1,387	S
B105	13	21'-4"	1,473	S
B1001	67	10'-8"	3,075	S

Mark	No.	Length	Weight	Shp.
FORWARD ABUTMENT				
A601	56	17'-2"	1,444	B
A602	14	17'-10"	375	B
A501	25	12'-10"	335	B
A502	10	36'-7"	382	S
A503	25	5'-8"	148	S
A504	18	6'-2"	309	B
A505	9	34'-5"	370	S
A506	4	15'-4"	66	S
P901	12	9'-8"	394	B
P902	20	17'-5"	1,184	S
P903	84	5'-7"	1,595	B
P904	32	17'-0"	1,850	S
P905	21	25'-7"	1,827	S
P906	21	47'-3"	3,374	S
P907	32	18'-9"	2,040	S
P601	8	33'-1"	398	S
P602	4	29'-2"	175	S
P501	222	8'-3"	1,910	B
P502	336	6'-8"	2,336	S



**MAHONING COUNTY
MAH - 18 - 15.50**

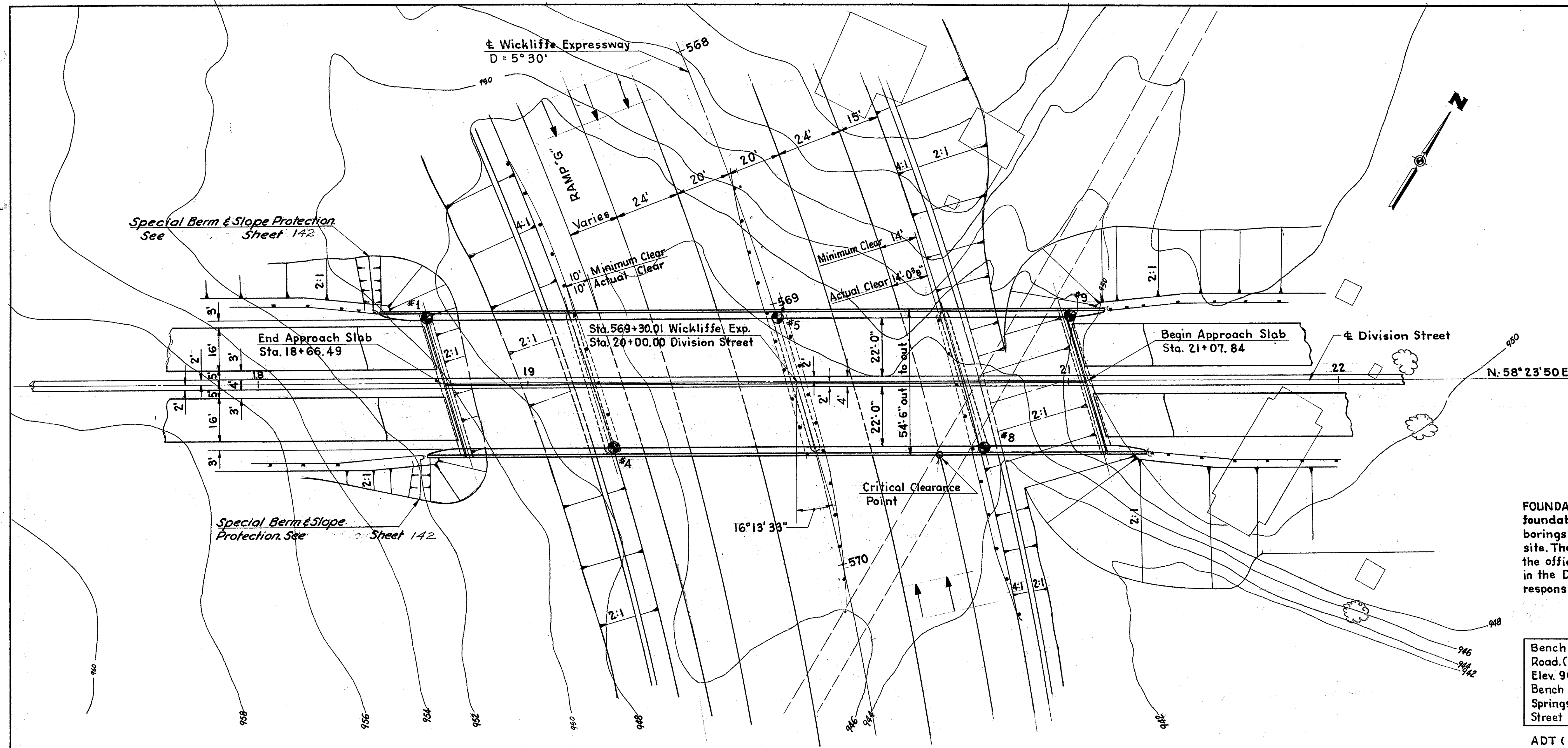
Mark	No.	Length	Weight	Shp.
REINFORCING STEEL LIST				
BRIDGE NO. MAH - 18 - 1629				
UNDER VESTAL ROAD				
MAHONING COUNTY STA. 559+80.95				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
S.J.F.	Dg		EED	L.G.H. 7-18-65

NOTES:

- Bar size is indicated in the bar mark. The first digit where three digits are used, and the first two digits where four are used, indicate the bar size number. For example, 5701 is a No. 7 size and B1001 is a No. 10 size.
- Spiral Reinforcing Bars: The "Length" shown in the steel list for the spiral bars is the distance from the top of the footing to the top of the vertical reinforcing bars. The No. of Turns shown is the "Length" divided by the pitch, plus three turns (total number of closed coils), expressed as the nearest whole number. Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item 5-4. 1/2 closed coils shall be provided at the ends of each spiral unit. Four steel channel, tee or angle spacers, weighing approximately 0.68 lbs. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing steel and is included in the tabulated quantity of spiral bars.

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MAH-18-15.50
MAHONING COUNTY



Special Berm & Slope Protection
See Sheet 142

Special Berm & Slope Protection
See Sheet 142

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

Bench Mark 210: N. Bolt on hydrant of 1206 Argo Road. (110' W. on Argo Street from Brockway Ave). Elev. 961.637.
Bench Mark 211: N. Bolt of hydrant N.E. corner Salt Springs Road and Argo Street (130' East on Argo Street from Salt Springs Road). Elev. 944.401.

ADT (1975) = 10,300 (15% Cl. B & C).

PROPOSED STRUCTURE
TYPE: Continuous Steel Beam with reinforced concrete deck and superstructure.
SPANS: 54'-0", 77'-2", 62'-0", 43'-6"
ROADWAY: 48'-0" $\frac{1}{2}$ " 2'-3" Safety Curbs including 4'-0" Raised Median.
LOAD FREQUENCY: CF 2000 (57) adequate for alternate A.A.S.H.O. Loading.
SKIEW: 16° 13' 33" R.F.
WEARING SURFACE: 1" Monolithic concrete.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Tangent

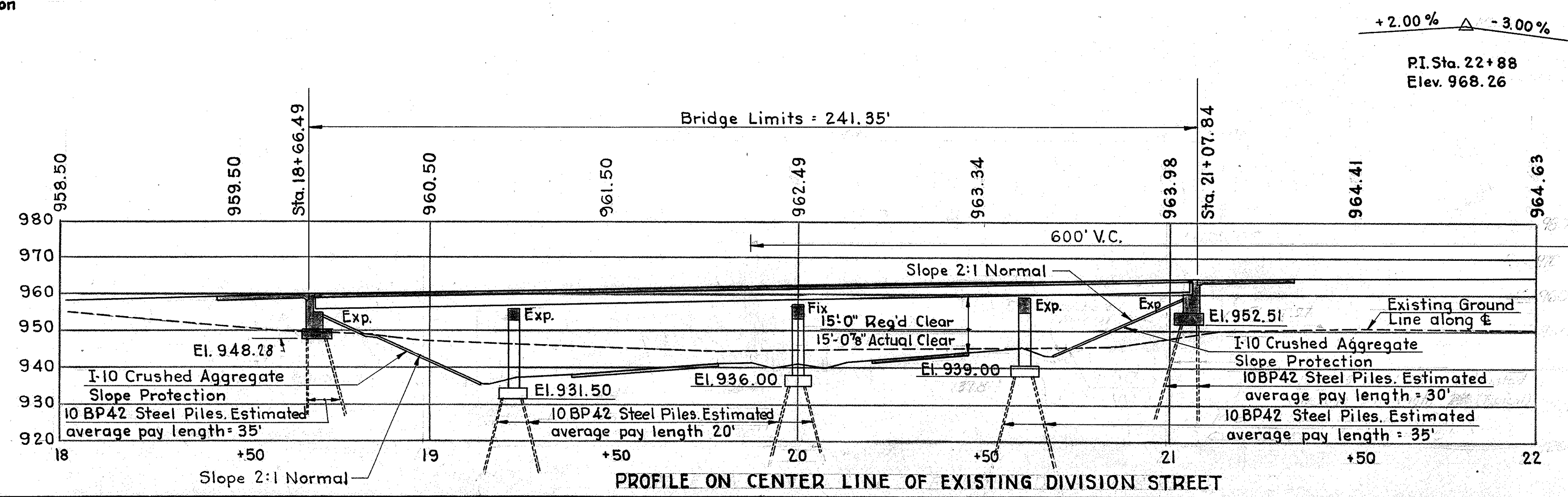
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

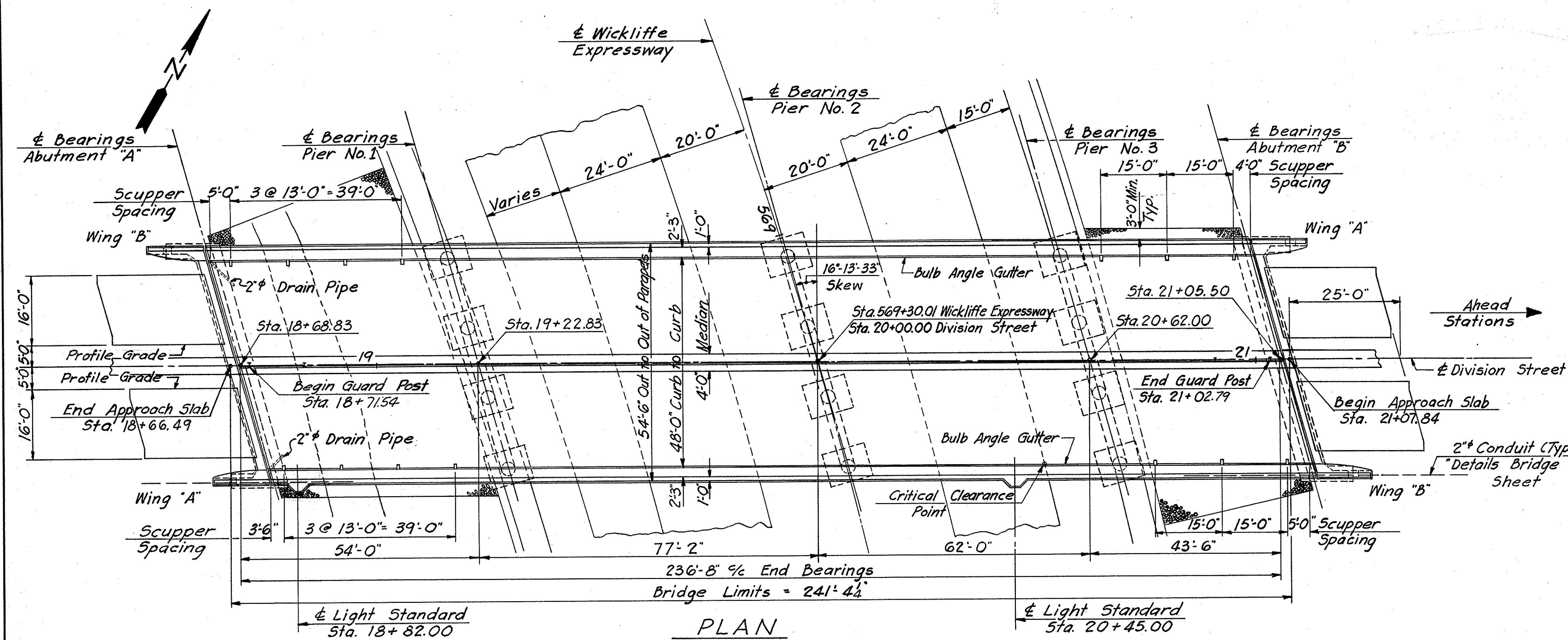
SITE PLAN
BRIDGE NO. MAH-18-1649
UNDER DIVISION STREET

MAHONING COUNTY STA. 569 + 30.01

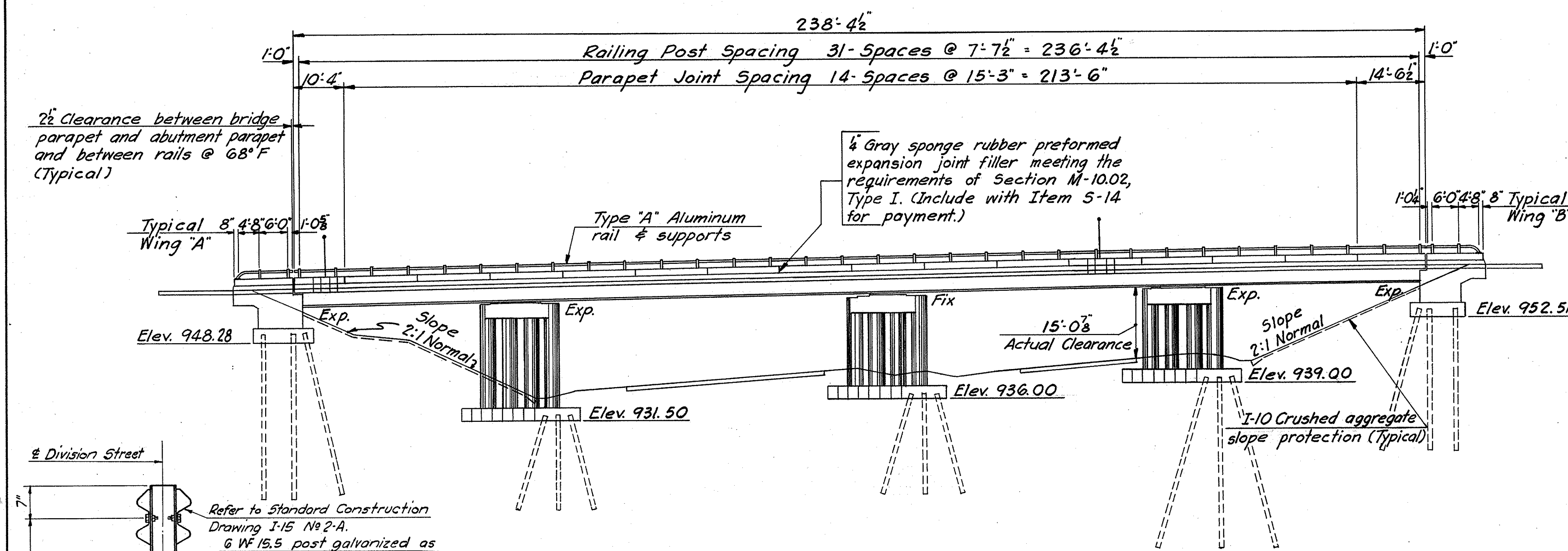
PRESENT TOPOGRAPHY SURVEYED	PROPOSED WORK DRAWN	DESIGNED	DRAWN	CHECKED	REVIEWED
	MM	JK	R.T.R.	E.F.R.	6-26-63

⊙ Indicates boring location



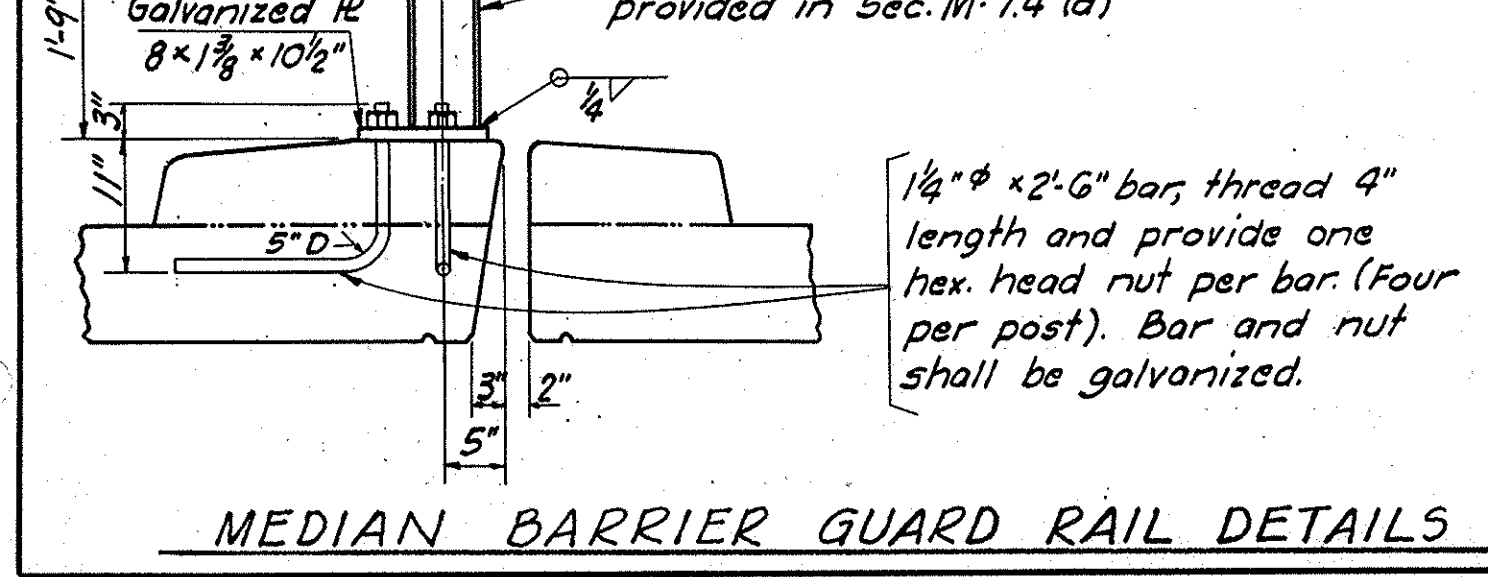


- GENERAL NOTES:**
- DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.
 - REFERENCE shall be made to Standard Drawings CSB-2-56, Sheets 2 of 6, revised 2-2-59, AR-1-57 revised 4-2-62; F5B-1-62 revised 1-15-63, AS-1-54 revised 7-5-62, and Supplemental Specifications 5-101 dated 7-12-62.
 - EXCAVATION QUANTITY includes the removal of fill material required for the construction of abutments.
 - PILES shall be driven to a minimum bearing capacity of 28 tons per pile for the abutments, and 33 tons per pile for the piers.
 - WELDING of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.
 - SURFACE FINISH OF CONCRETE: The requirements of Sec. 5-120, Rubbed Finish, shall apply to the following exposed surfaces:
 - The entire superstructure except top/bottom surfaces of sidewalks and roadways.
 - The entire surface of piers, abutments, and parapets except bridge seats and backwalls.
 - REFERENCE shall be made to Supplemental Specification 5-307 revised 10-1-64 and Standard Drawing SD-2-64 dated 11-25-64.
 - DESIGN LOADING: CF2000(57)
 - CONCRETE CLASS "C": Basic unit stress 1333 psi.
 - CONCRETE CLASS "E": Basic unit stress 1133 psi.
 - STRUCTURAL STEEL: ASTM A36 - basic unit stress 20,000 psi. (ASTM A7 and A373 steel not permitted)
 - REINFORCING STEEL: ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 psi. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 psi.



ITEM	TOTAL	UNIT	DESCRIPTION	SUPERSTR	ABUTS.	PIERS	GENERAL
E-2	534	Cu.Yds	Unclassified excavation		2.94	240	
S-1	431	Cu.Yds	Class "C" concrete, superstructure	431			
S-1	111	Cu.Yds	Class "C" concrete, pier caps & columns			111	
S-1	117	Cu.Yds	Class "E" concrete, abutments above footings		117		
S-1	187	Cu.Yds	Class "E" concrete, footings		94	93	
S-14	233.79	Lin.Ft	Barrier guard rail, steel beam type (deep) including anchorage	233.79			
S-4	160,879	Lbs.	Reinforcing steel	115,350	14,749	30,780	
S-7	347,800	Lbs.	Structural steel	347,800			
S-8	347,800	Lbs.	Field painting of structural steel	347,800			
S-14	526.23	Lin.Ft.	Railing Type "A" (Alum. railing & supports, & conc. parapets)	476.75	49.48		
S-16	Lump Sum		First test pile				L.S.
S-18	2,775	Lin.Ft.	Steel piles 10 BP 42		975	4,800	
S-25			Electric lighting system complete				
S-29	40	Cu.Yds	Porous Backfill		40		
S-29	14	Each	Scuppers, including supports	14			
I-10	687	Sq.Yds	Crushed aggregate slope protection				687
S-101	431	Each	Water-reducing, set-retarding admixture	431			

* See General Summary - Lighting Sheet.



MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

GENERAL PLAN & ELEVATION

BRIDGE NO. MAH-18-1649
UNDER DIVISION STREET

MAHONING COUNTY STA. 569 + 30.01

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
U.C.	U.C.	R.C.	Office	JK	8-26-63	

MAHONING COUNTY
MAH-18-15.50

NOTES:
 • PROCEDURE: The embankment shall be placed and compacted up to the finished spill-thru slope after which excavation shall be made for the abutment and piles driven.

• POROUS BACKFILL, 2 ft. thick, full length of abutment and wings shall extend up to the underside of the approach or to the finished ground surface. Excavation therefore, in excess of that required for construction of the abutment, shall be considered as paid for in the bid price per cubic yard paid for porous backfill.

• BRIDGE SEAT REINFORCING: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.

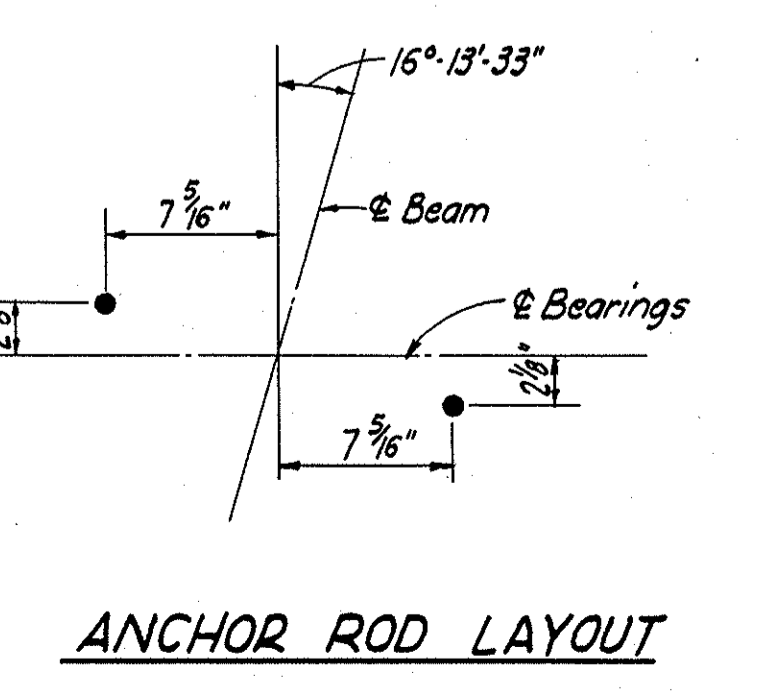
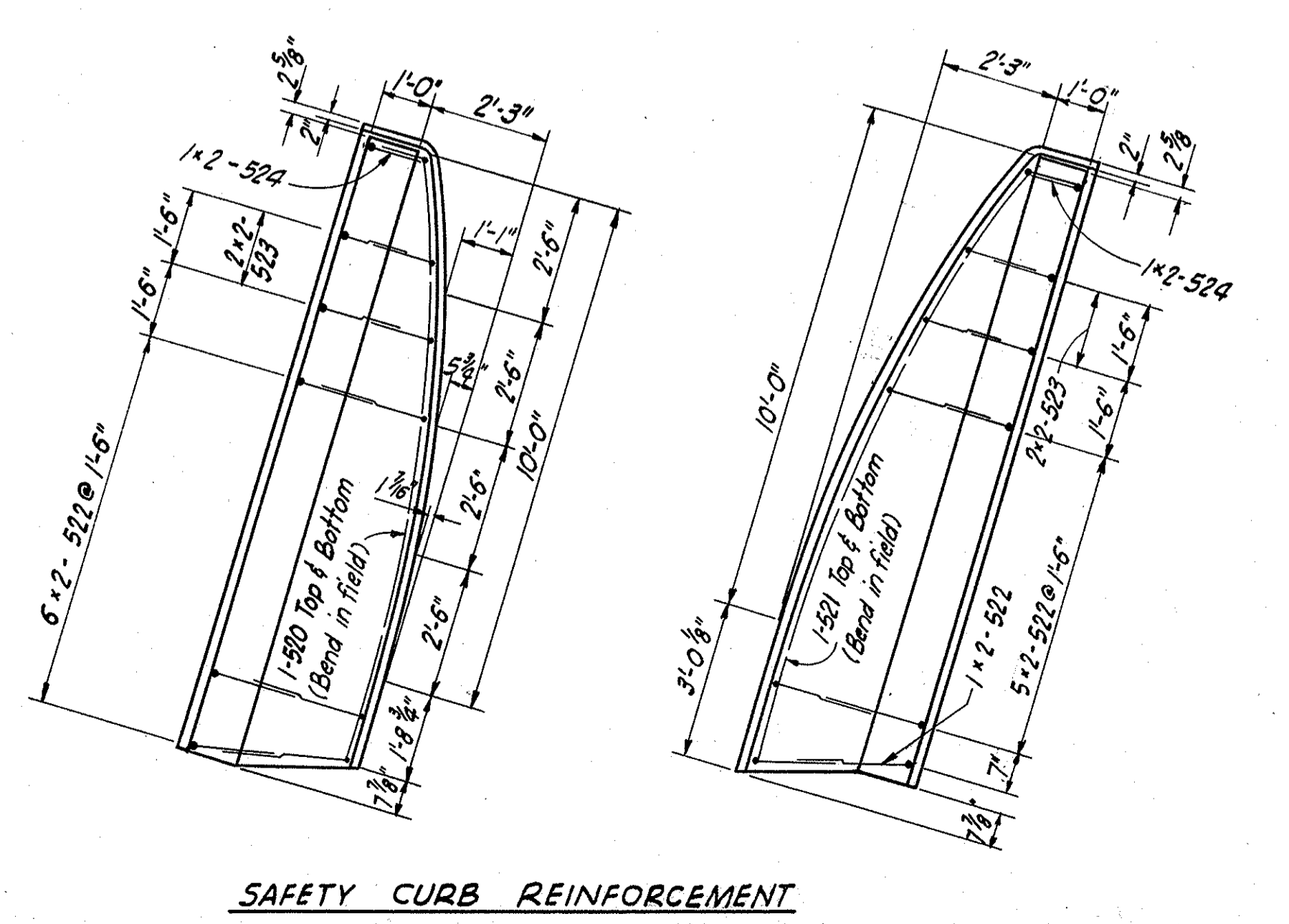
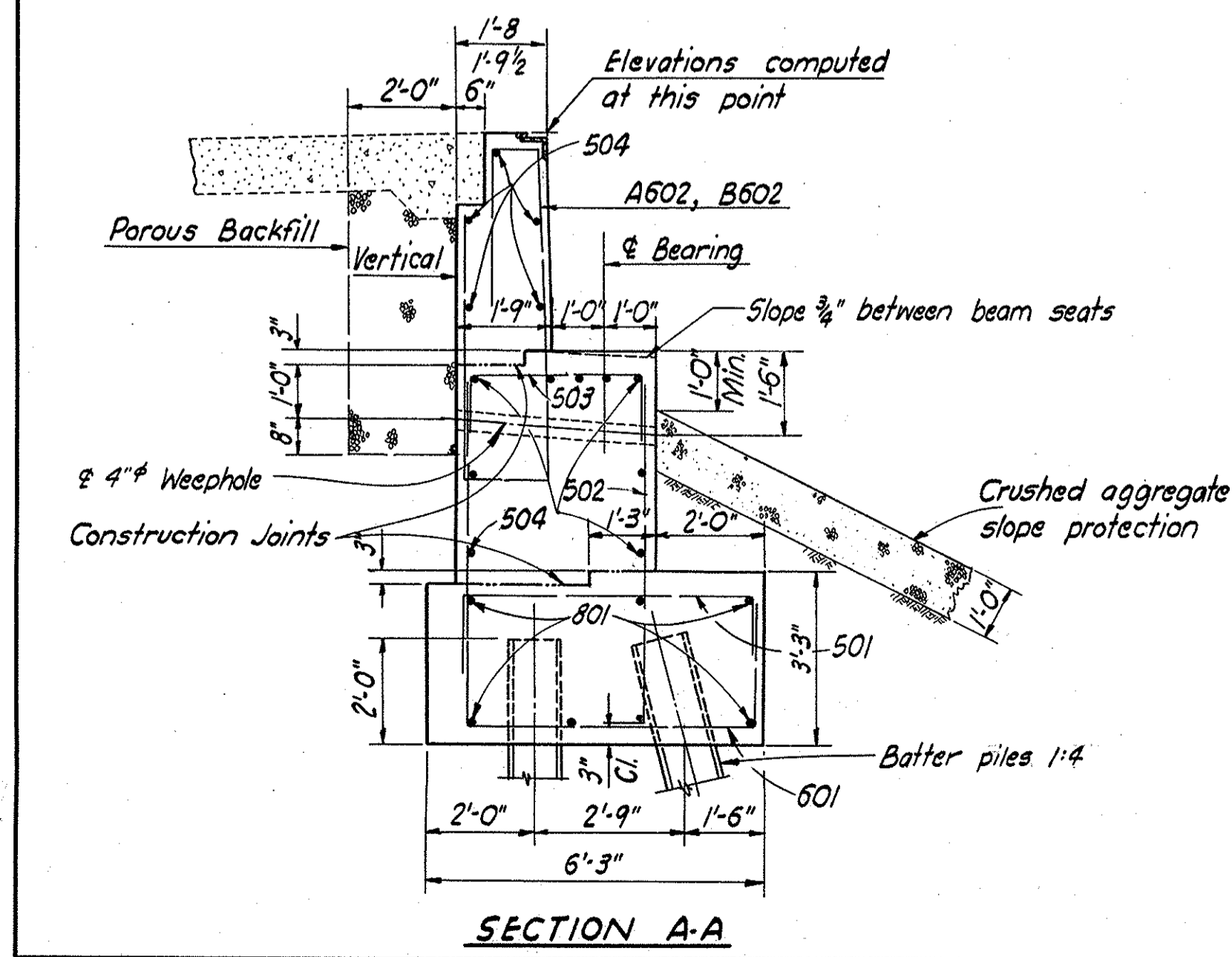
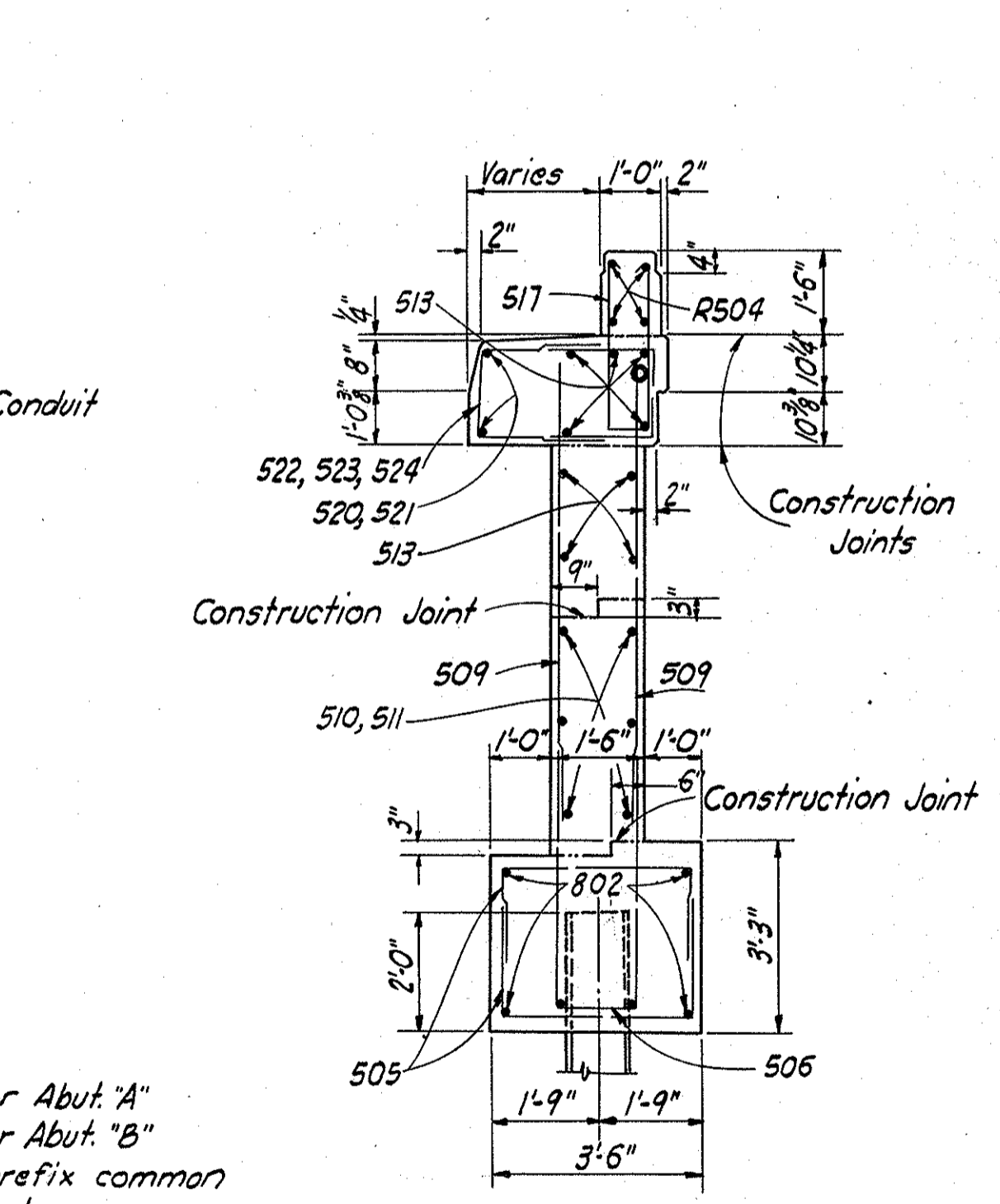
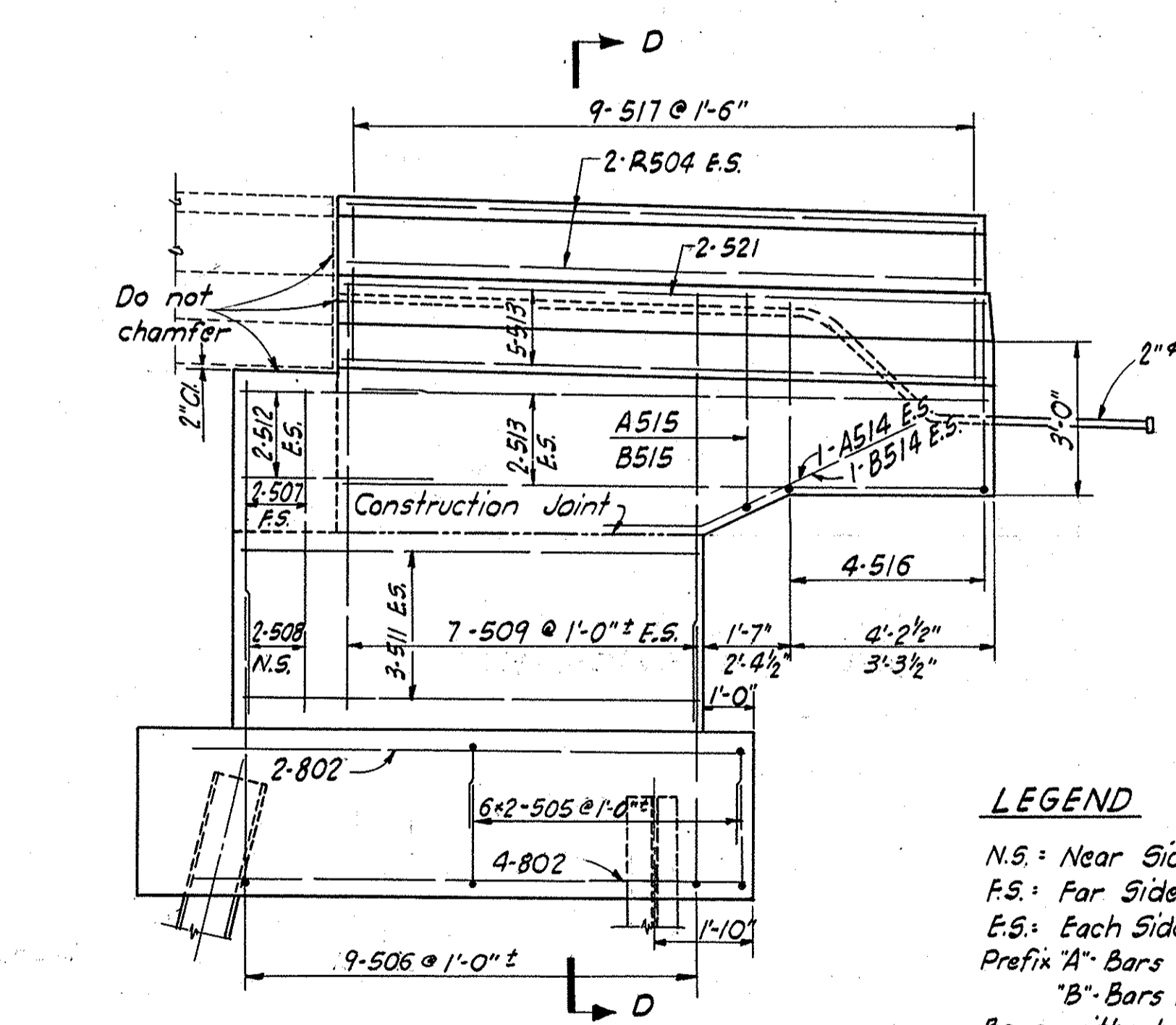
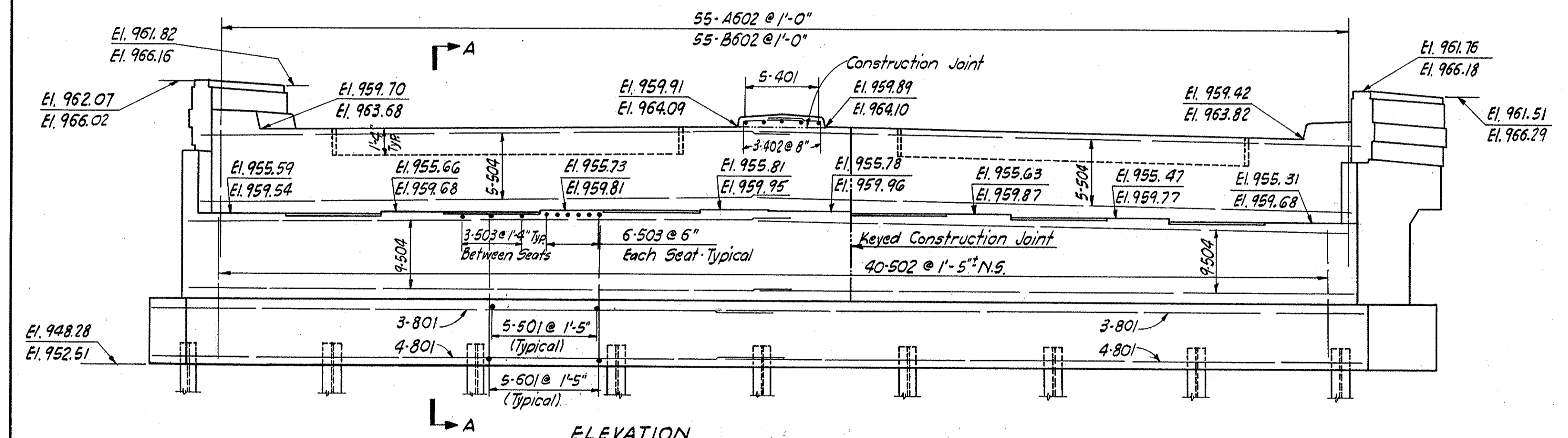
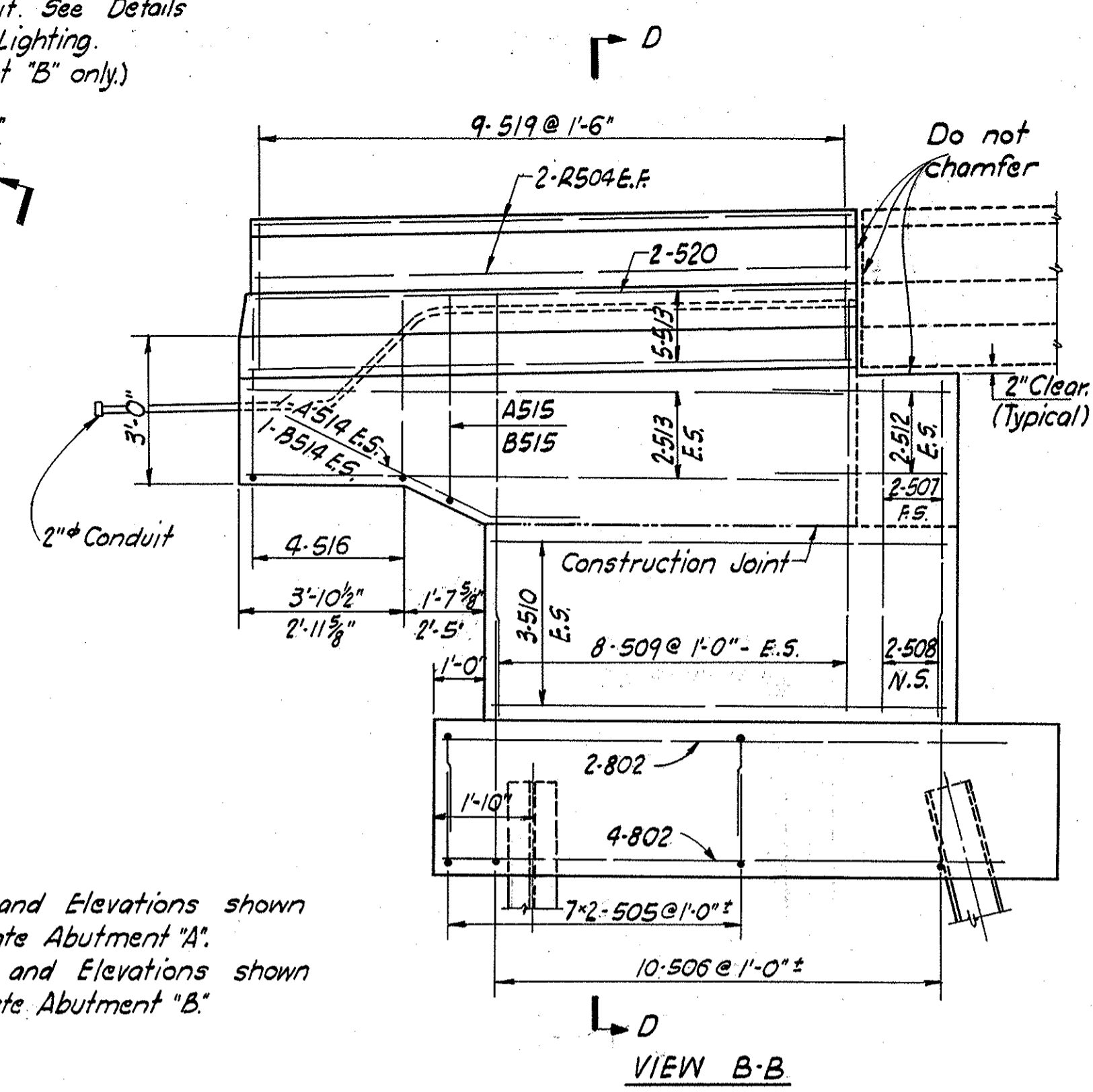
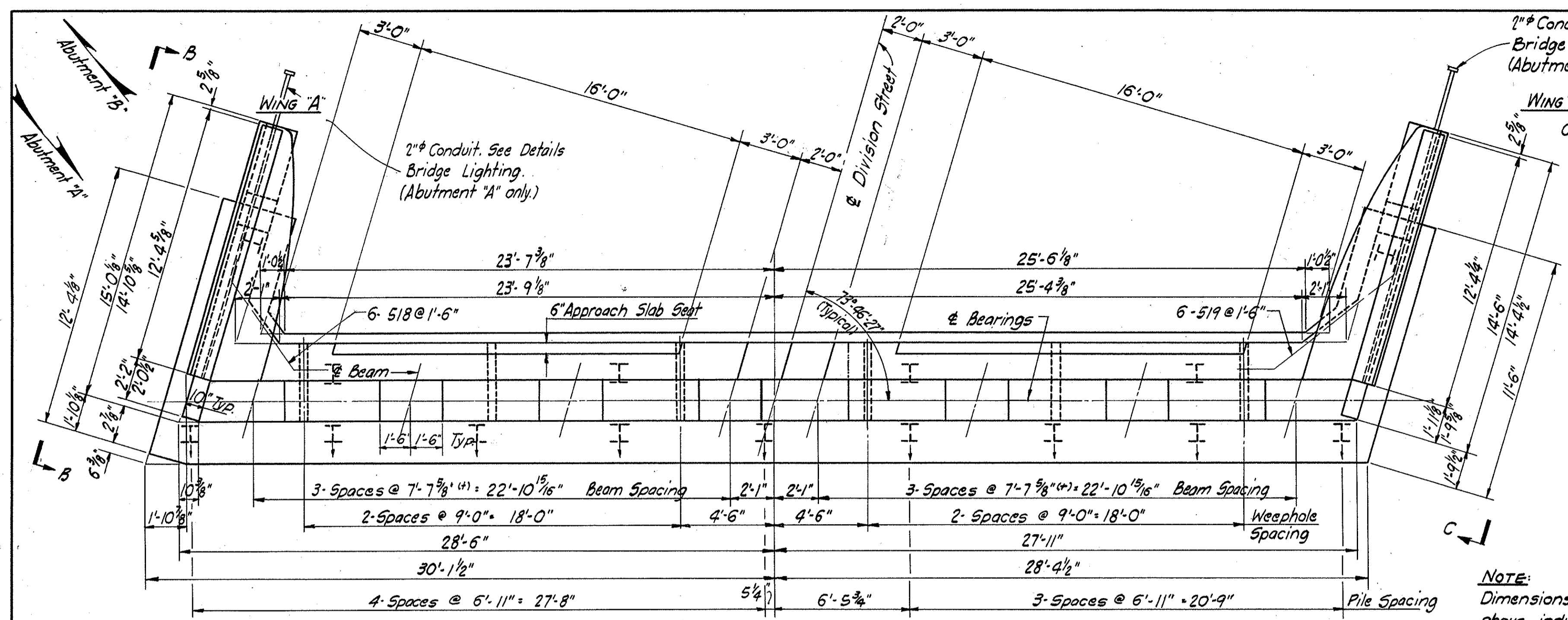
• Refer to Standard Drawing AR-1-57 (Revised 4-2-62) for details of Type "A" railing and Concrete Parapet.

• All abutment parapet railings shall be continuous through railing posts.

• All abutment concrete shall be Class "E".

• All parapet concrete shall be Class "C".

• Concrete and reinforcing steel above parapet construction joint included with railing 5-14 for payment.



LEGEND
 N.S. - Near Side
 F.S. - Far Side
 E.S. - Each Side
 Prefix "A" - Bars for Abut "A"
 "B" - Bars for Abut. "B"
 Bars without prefix common to both abutments.

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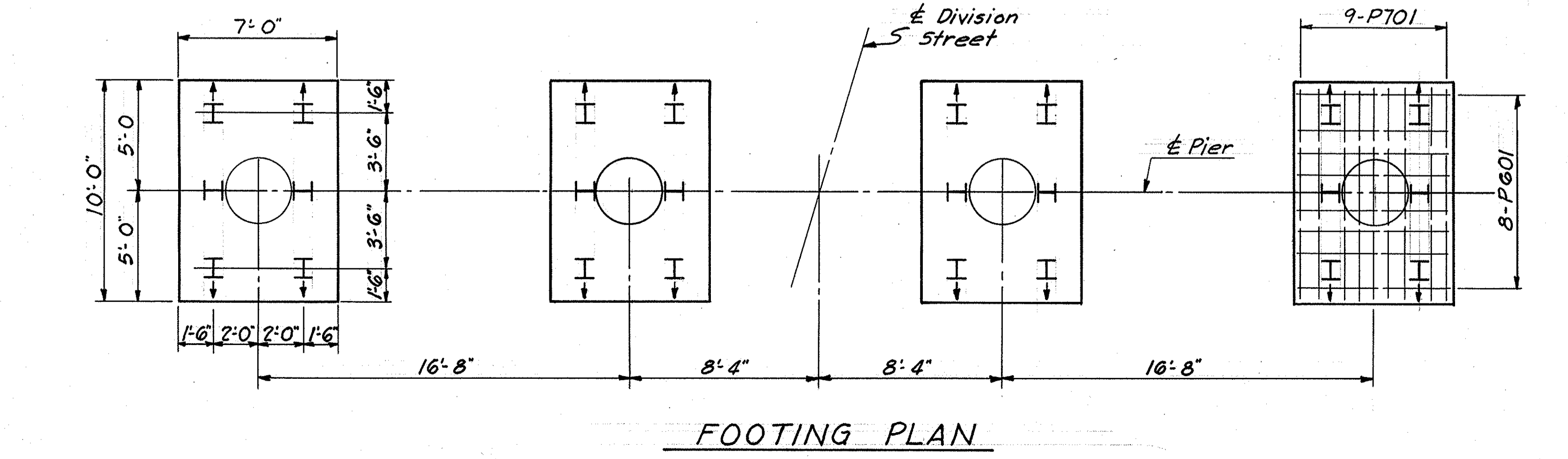
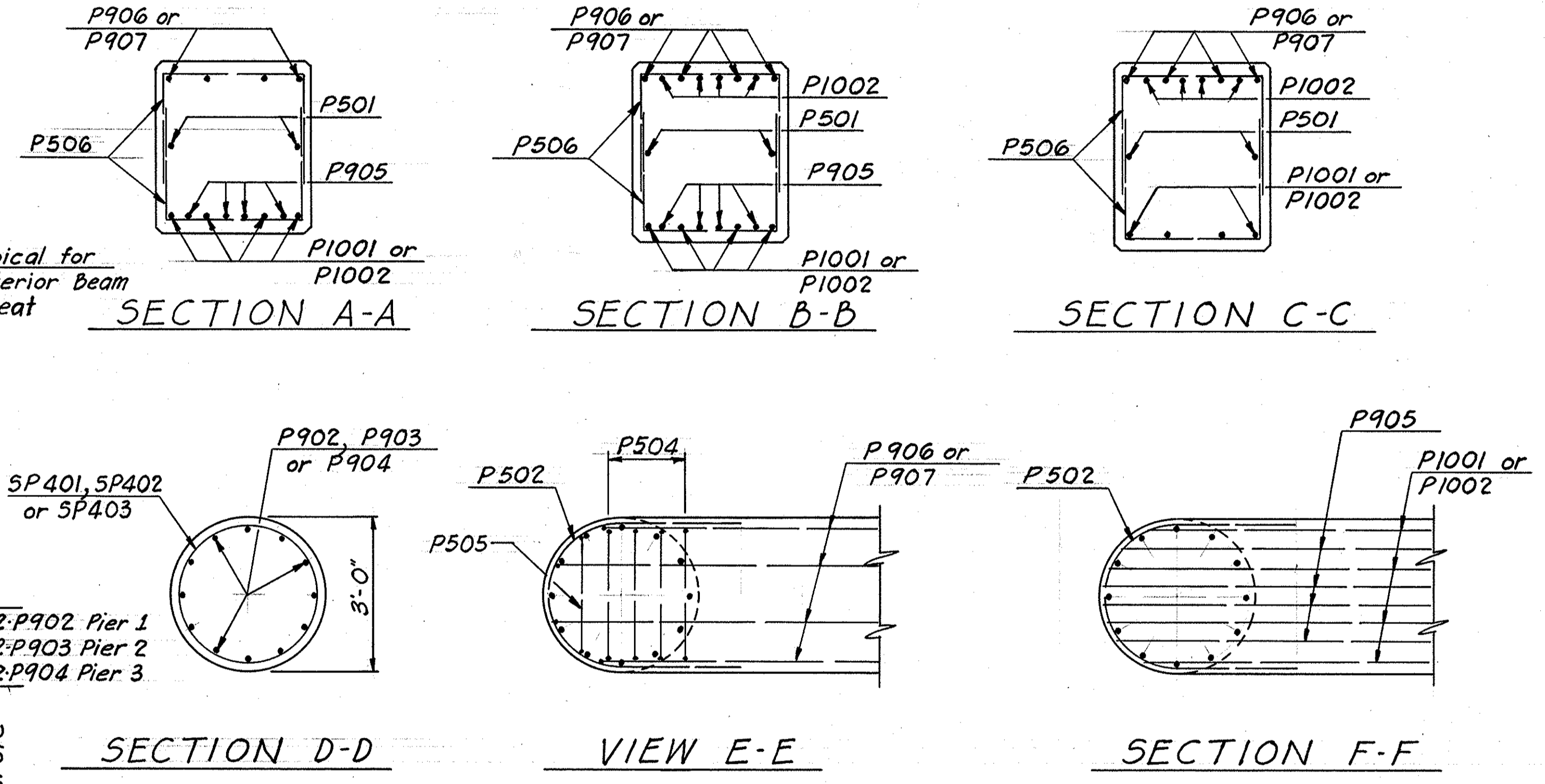
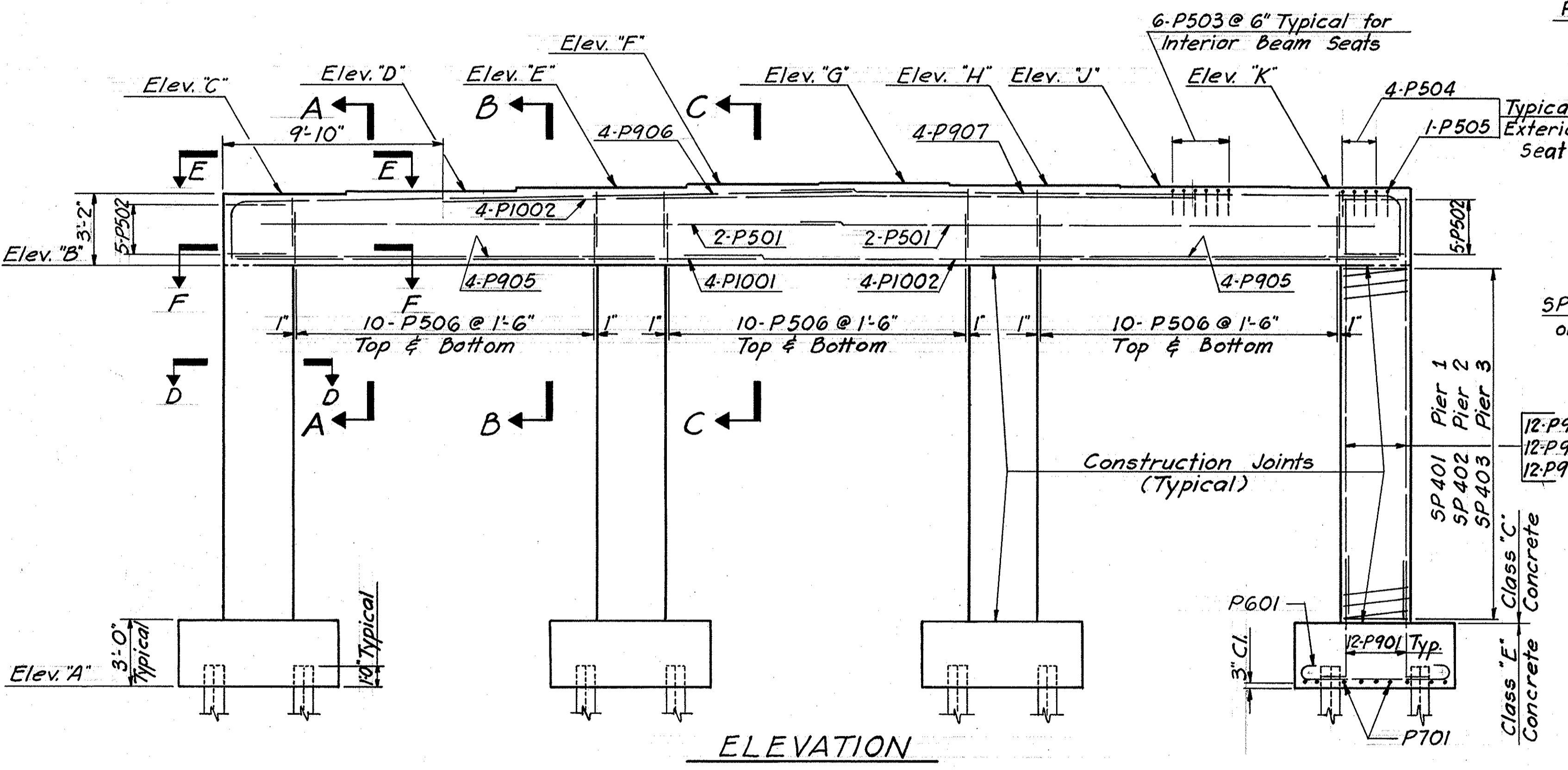
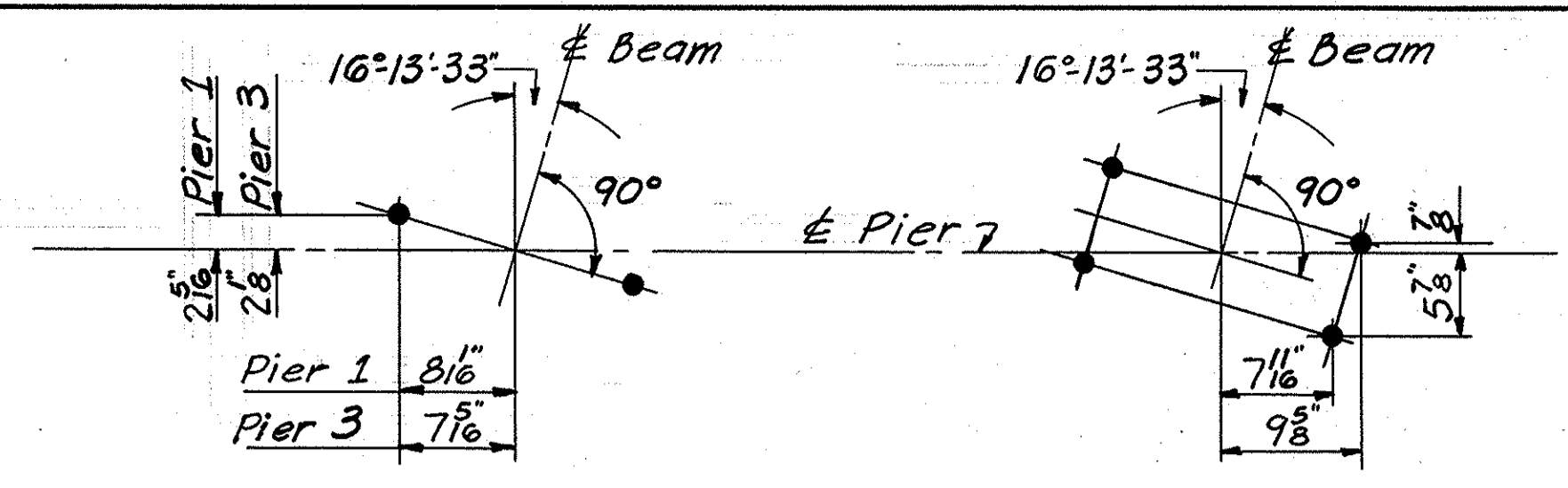
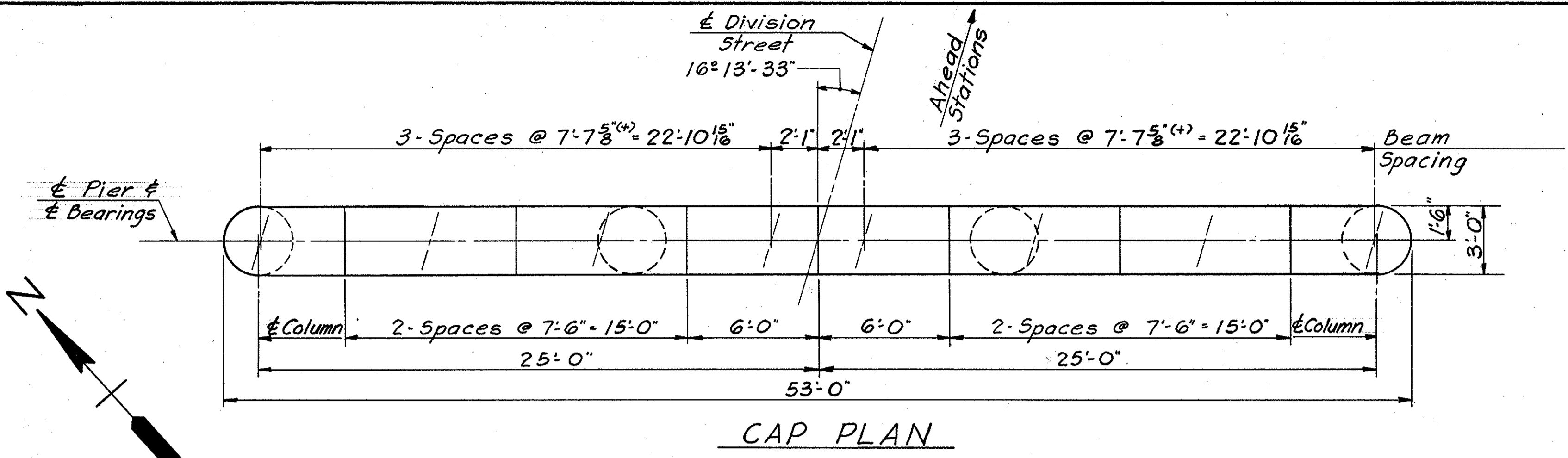
ABUTMENTS
 BRIDGE NO. MAH-18-1649
 UNDER DIVISION STREET

MAHONING COUNTY STA. 569 + 30.01

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised

8-26-63

MAHONING COUNTY
MAH-18-15.50



PIER ELEVATIONS			
Elevation	Pier 1	Pier 2	Pier 3
"A"	931.50	936.00	939.00
"B"	953.17	954.42	955.82
"C"	.35 956.59	957.59	958.99
"D"	.51 956.55	957.75	959.13
"E"	.67 956.71	957.91	959.28
"F"	.82 956.86	958.06	959.42
"G"	.85 956.89	958.08	959.44
"H"	.78 956.82	958.01	959.35
"J"	.10 956.74	957.93	959.27
"K"	.63 956.67	957.86	959.18

NOTES:

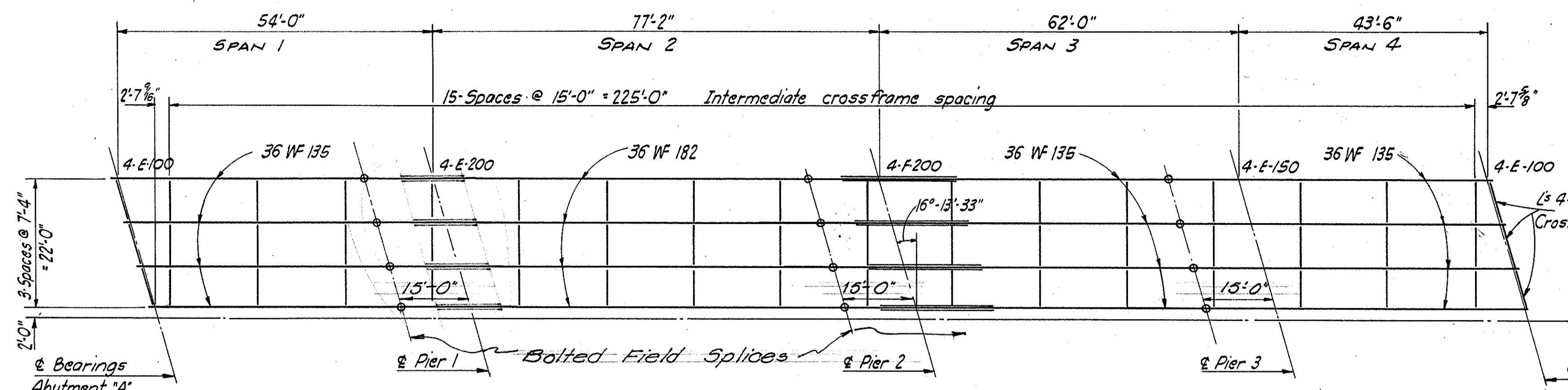
- Piles shown thus ↑ are to be battered 1:4 in direction indicated by arrow.
- Super-Structure Ground-A No. 1/0 AWG 7 strand soft annealed bare copper cable shall be encased in the outside column of pier No. 2. Connect (by Exothermic weld process) lower end of cable to a steel pile. Extend cable (in one continuous length) through top of pier with lead of sufficient length to exothermic weld upper end of cable to outside beam of super-structure.
- Payment for electrical grounds is included in the lump sum bid for 5-25 "Electrical Lighting System Complete"

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ROCHESTER, PENNSYLVANIA

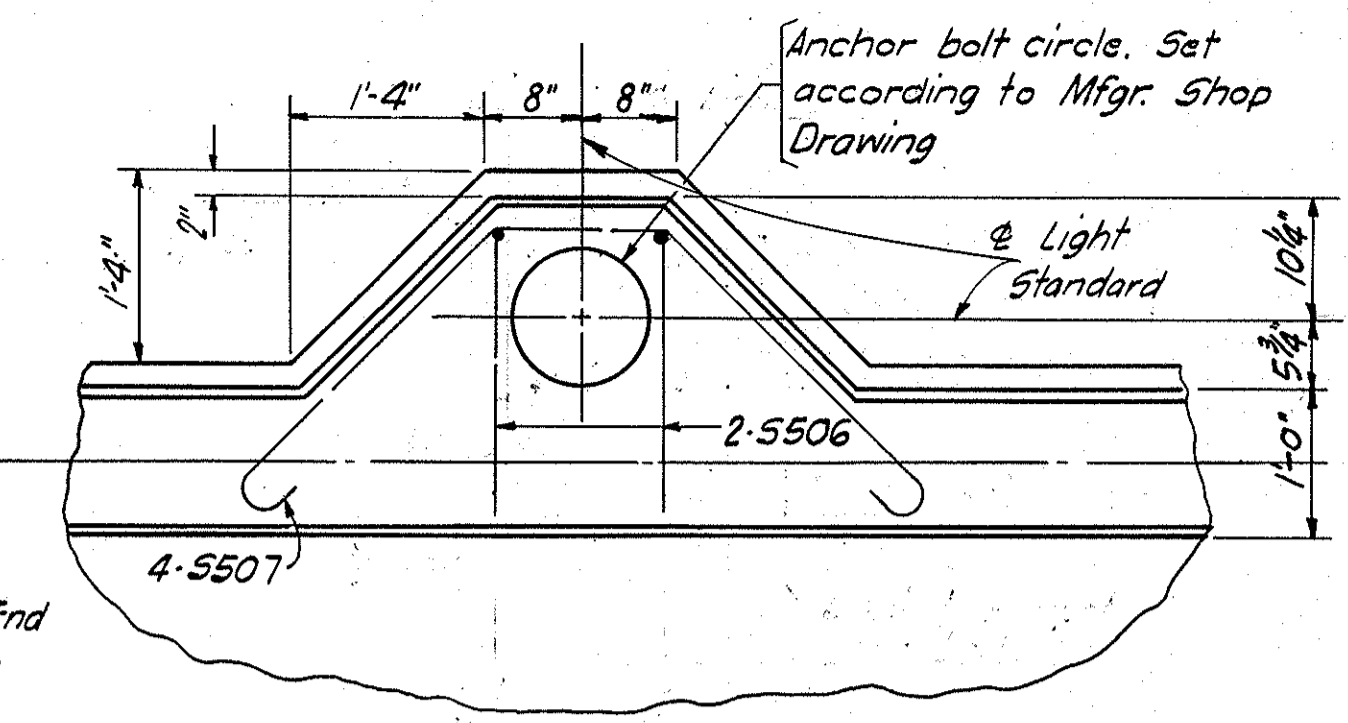
PIERS
BRIDGE NO. MAH-18-1649
UNDER DIVISION STREET

MAHONING COUNTY	STA. 569 + 30.01					
Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
	R.C.	R.C.			8-26-65	12-22-65

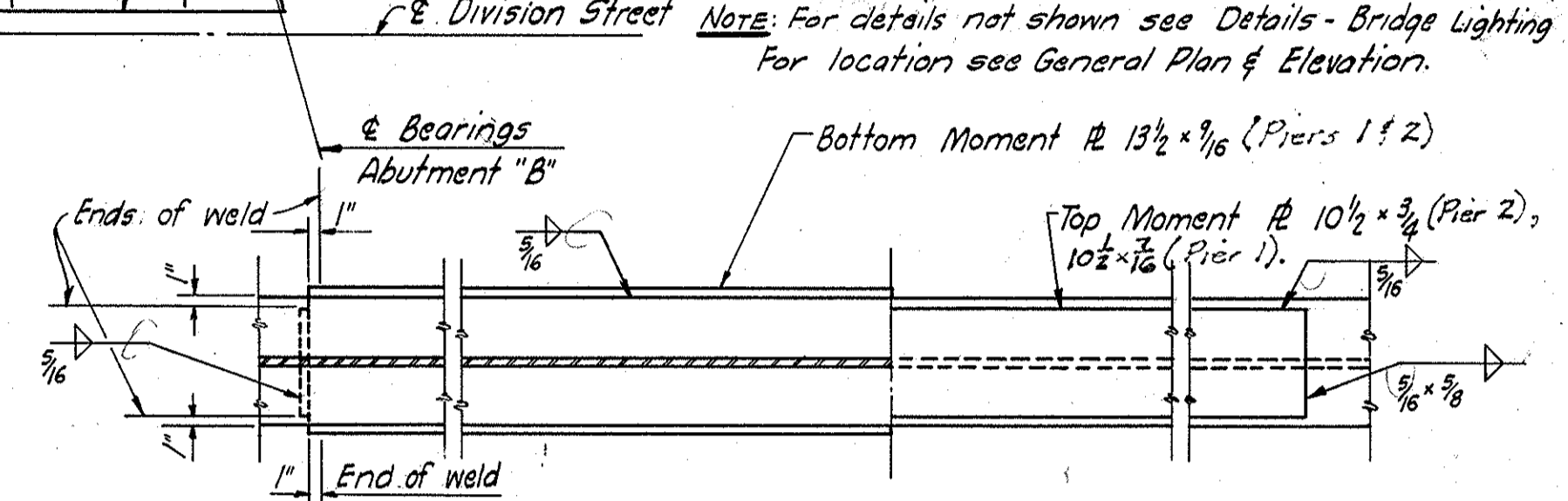
MAHONING COUNTY
MAH-18-15.50



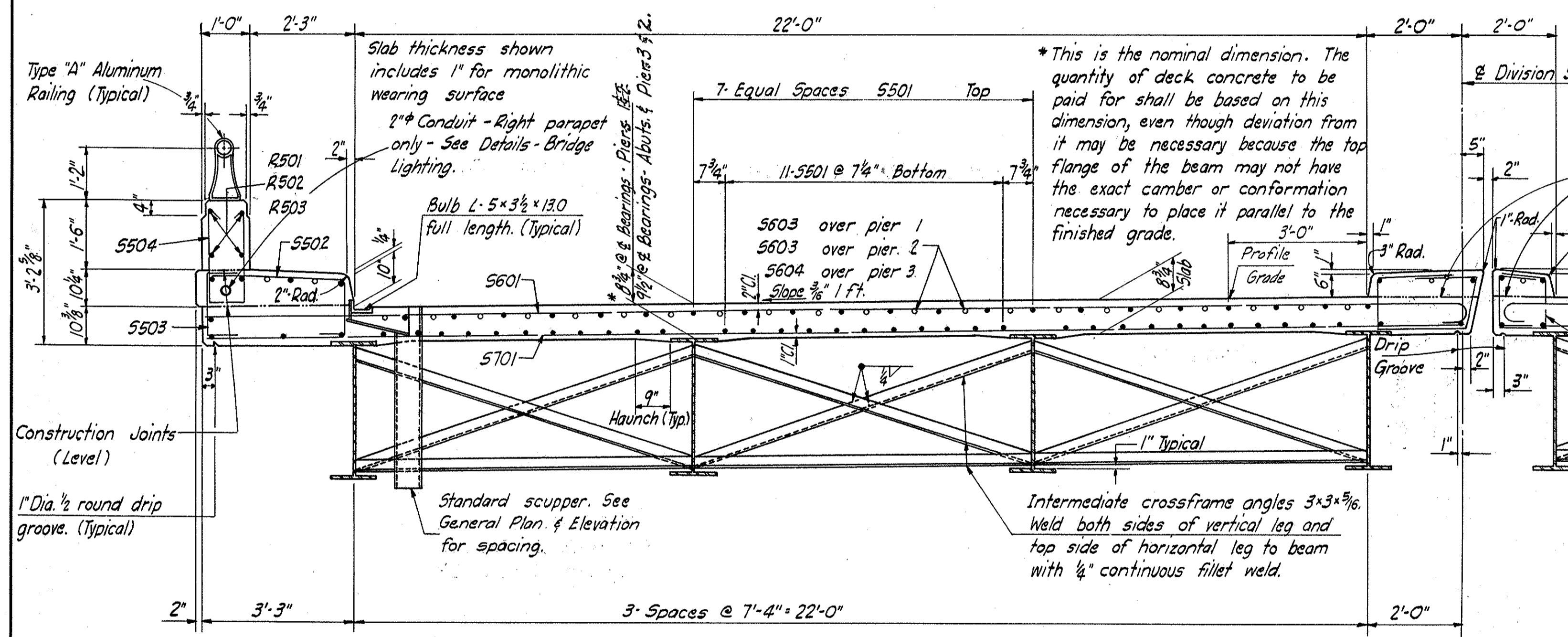
STEEL FRAMING PLAN
Left half shown - Right half similar



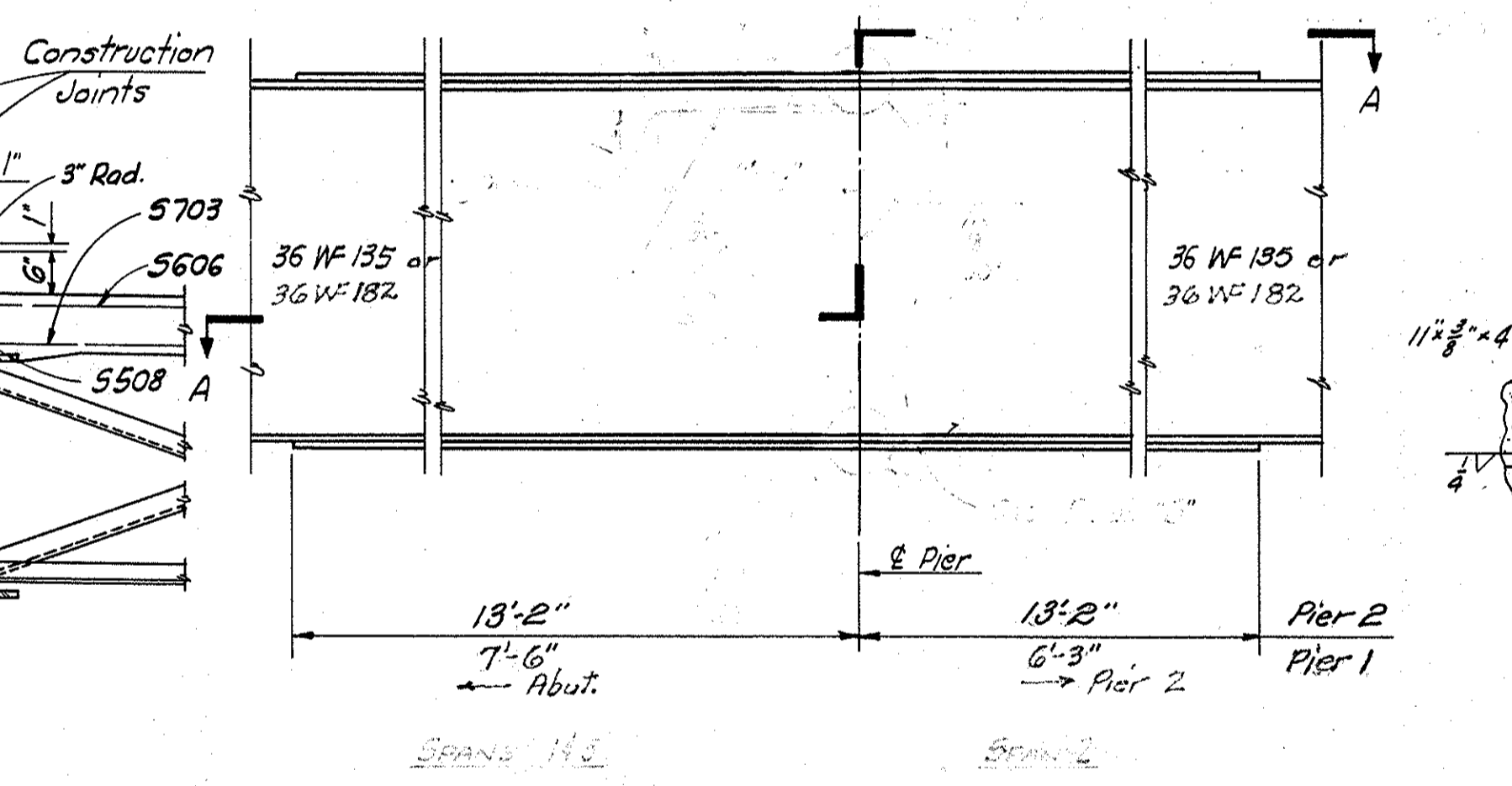
PLAN - LIGHT STANDARD BASE



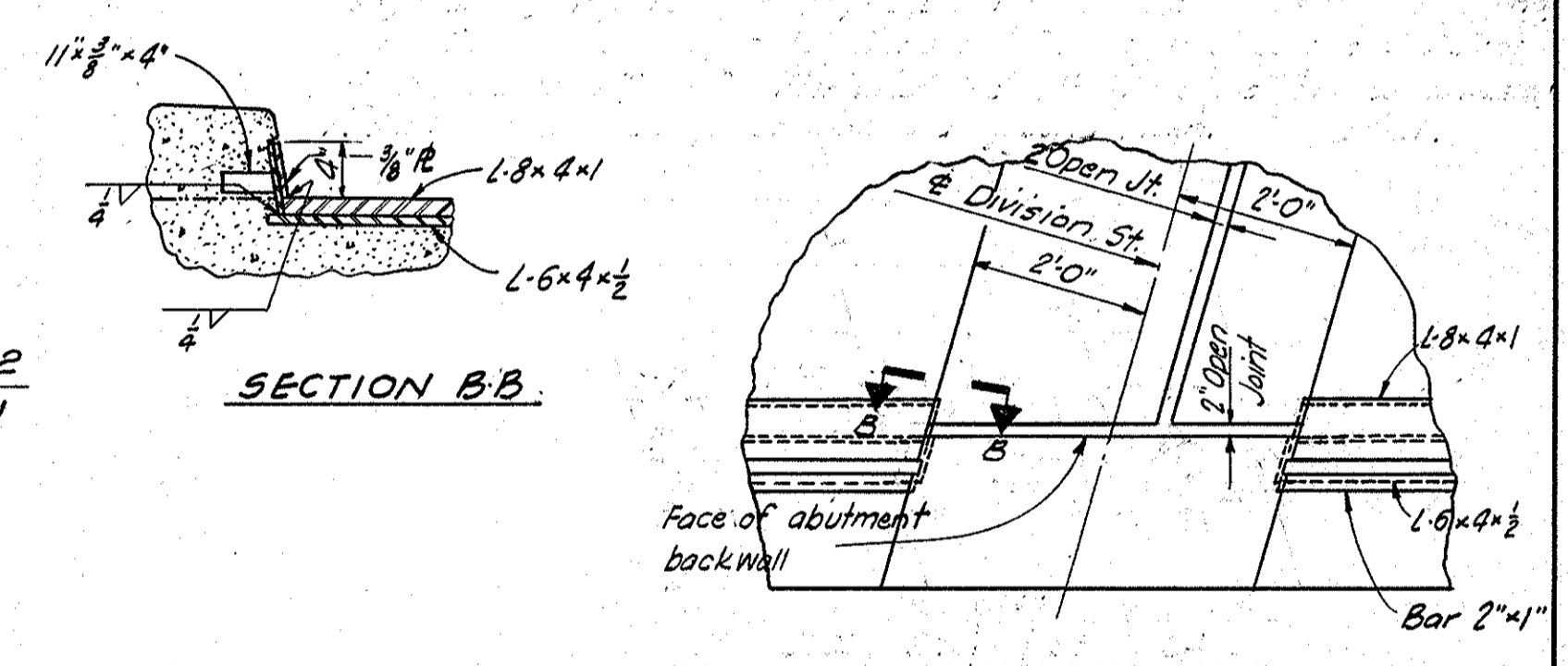
SECTION A-A



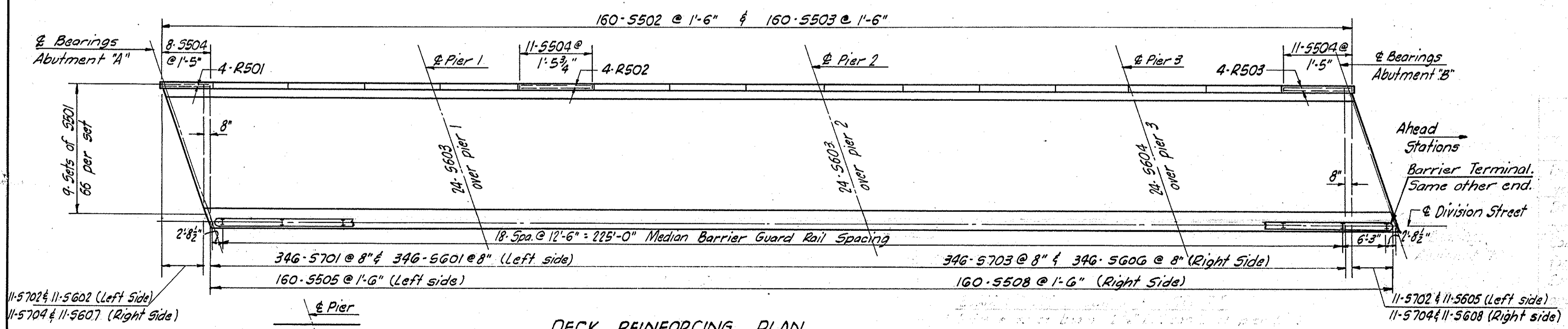
PART CROSS SECTION
Left side shown - Right side similar



BEAM DETAILS AT PIERS 1 & 2



END FINISH DETAIL AT MEDIAN



DECK REINFORCING PLAN
Left side shown - Right side similar

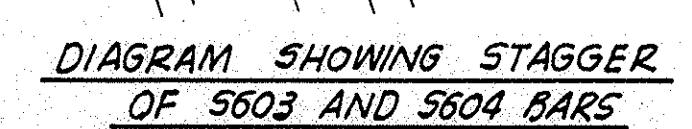
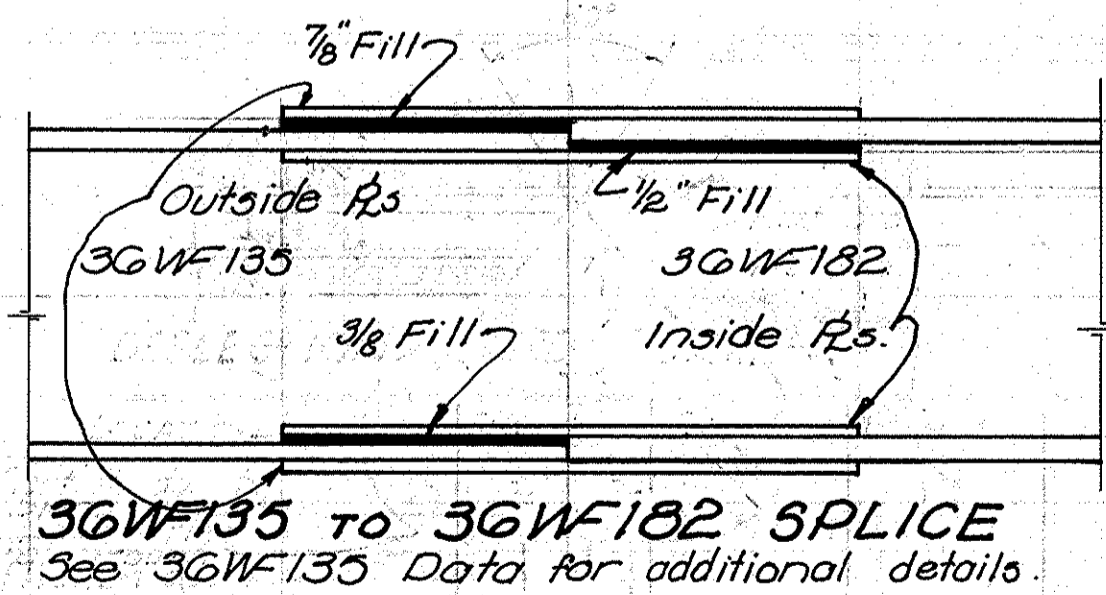


DIAGRAM SHOWING STAGGER OF 5603 AND 5604 BARS



36WF135 to 36WF182 SPLICE
See 36WF135 Data for additional details.
For BOLTED FIELD SPLICE DETAILS see Std. Dwg. SD-2-G4

- NOTES:**
- Refer to Standard Drawing AR-1-57 for details of Type "A" aluminum railing and concrete parapet; to Standard Drawing CSB-2-56, Sheets 2&3 of 6 for details of expansion dams, end crossframes, 2" pipe drains, curb plates and scuppers; and to Standard Drawing FSB-1-62 for bearings.
 - Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
 - See General Plan and Elevation for location of scuppers, 2" pipe drains and railing posts.
 - CONCRETE DECK PLACING:** In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress up grade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
 - DECK SLAB HAUNCH:** The haunch in the deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" except that the maximum slope shall not exceed 3" per foot. Payment for deck slab concrete shall be based on the 9" width.
 - MACHINE FINISH:** The concrete bridge deck shall be finished by the use of a finishing machine.

- All concrete shall be Class "C."
- ELECTRICAL LIGHTING SYSTEM:** For additional details refer to drawings "Details - Bridge Lighting" and "General Lighting Notes".

DEFLECTION AND CAMBER				
LOCATION	SPAN 1	SPAN 2	SPAN 3	SPAN 4
Deflection due to weight of steel	0"	3/16"	0"	0"
Deflection due to remaining dead load	1/4"	3/16"	1/4"	1/8"
Convexity required for vertical curve	0"	0"	1/2"	1/4"
Sum of deflections and convexity	1/4"	1"	3/4"	3/8"
Required Camber	0"	1"	3/4"	0"

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS
BRIDGE NO. MAH-18-1649
UNDER DIVISION STREET

MAHONING COUNTY			STA. 569 + 30.01		
Designed	Drawn	Traced	Checked	Reviewed Date	Revised
EB	mm	mm	EFR	8-20-63	12-22-65

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

239

**MAH-18-15.50
MAHONING COUNTY**

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

BENCH MARK #211 Elev. 944.38 North Bolt of hydrant N.E. corner Salt Springs and Argo. 130'E. on Argo from Salt Springs.

BENCH MARK #212 Elev. 928.66 Top stem of hydrant N.E. corner Division and Salt Springs. 35'E. on Division from Salt Springs.

ADT (1975) = 13,195

DIVISION STREET EXPRESSWAY OVER SALT SPRINGS ROAD

TYPE: Continuous steel beam with reinforced concrete deck and substructures.
SPANS: 46'-6", 66'-8", 46'-6"
ROADWAY: Varies; 72'-8" to 63'-8"
LOAD FREQUENCY: C.F. 2000 (57). Adequate for alternate A.A.S.H.O. loading.
SKEW: 16°03'48" R.F.
WEARING SURFACE: 1" Monolithic concrete.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Tangent and spiral.
SUPERELEVATION: Varies

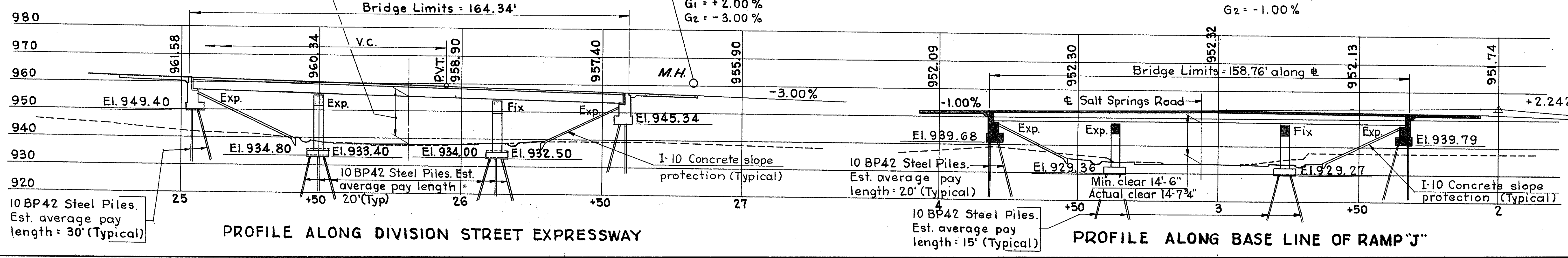
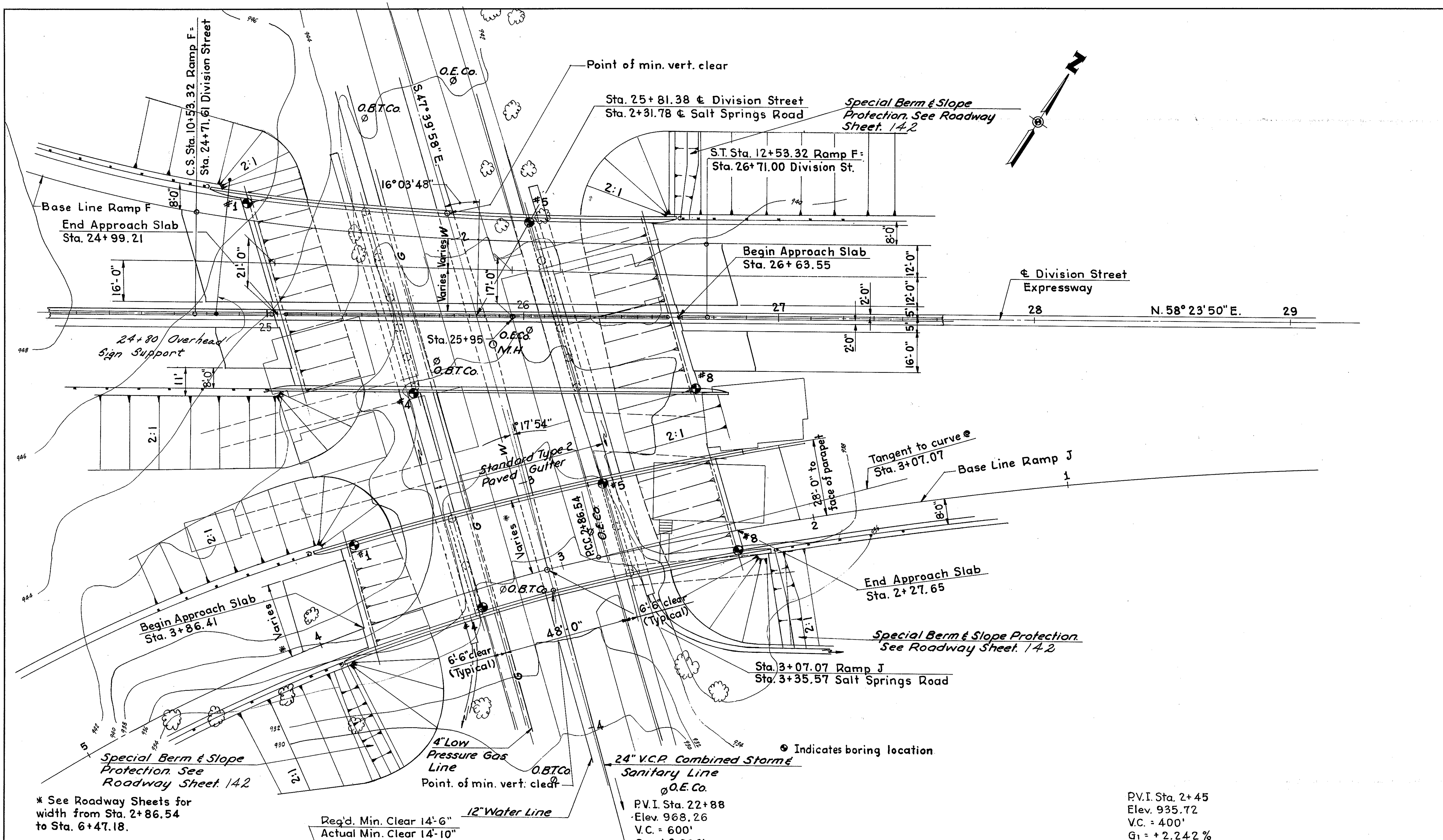
ADT (1975) = 6130

RAMP J OVER SALT SPRINGS ROAD

TYPE: Continuous steel beam with reinforced concrete deck and substructures.
SPANS: 45'-0", 64'-0", 45'-0"
ROADWAY: Varies; 33'-8" to 36'-10"
LOAD FREQUENCY: C.F. 2000 (57). Adequate for A.A.S.H.O. alternate loading.
SKEW: 1°17'54" R.F. to the local tangent.
WEARING SURFACE: 1" Monolithic concrete.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Curve
SUPERELEVATION: Varies.

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ROCHESTER, PENNSYLVANIA

SITE PLAN						
BRIDGE NO MAH-18-RAMP J						
OVER SALT SPRINGS ROAD						
MAHONING COUNTY						STA. 3+07.07
Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
EFR	EFR	R.T.R.	R.A.L.	SK	8-26-63	



Add Traffic Count

MAHONING COUNTY
MAH-18-15.50

GENERAL NOTES:

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.

REFERENCE shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59; AR-1-57 revised 4-2-62; F5B-1-62 revised 1-15-63; AS-1-54, revised 7-5-62 and Supplemental Specifications 5-101 dated 7-12-62

EXCAVATION QUANTITY includes the removal of fill material required for the construction of abutments.

PILES: All piles to be 10 BP42 steel piles. Piles shall be driven to a minimum bearing capacity of 31 tons per pile for the abutments and 35 tons per pile for the piers.

WELDING of structural steel shall be Class A except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.

CURBS: FINISH SURFACES: The requirements of Sec. 3-1.21, Subbed Finish, shall apply to the following exposed surfaces:

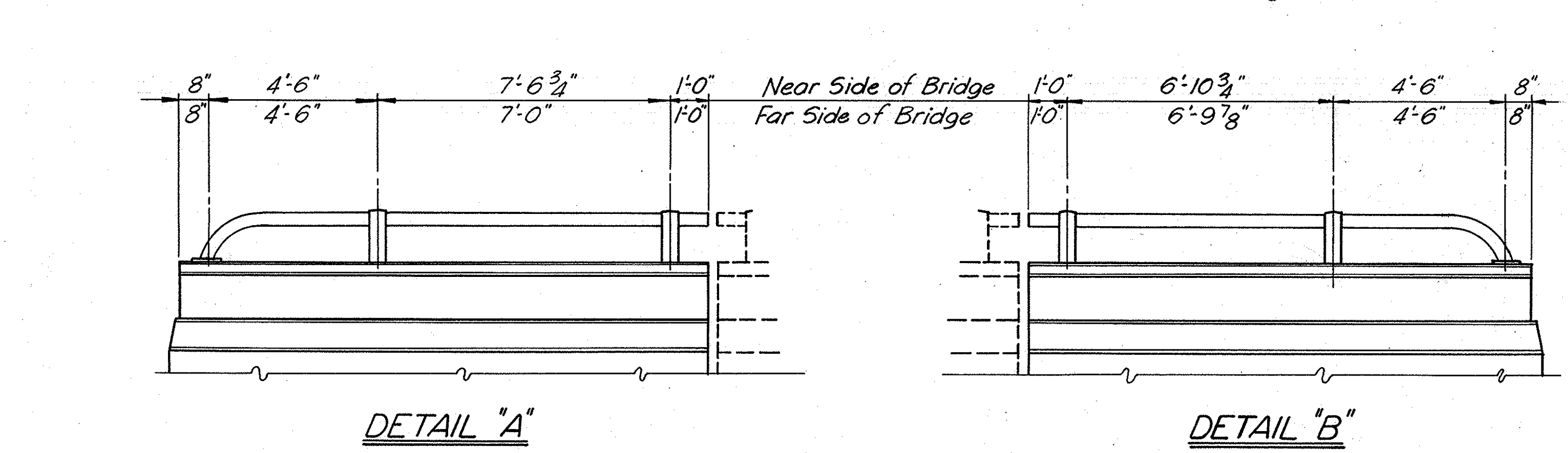
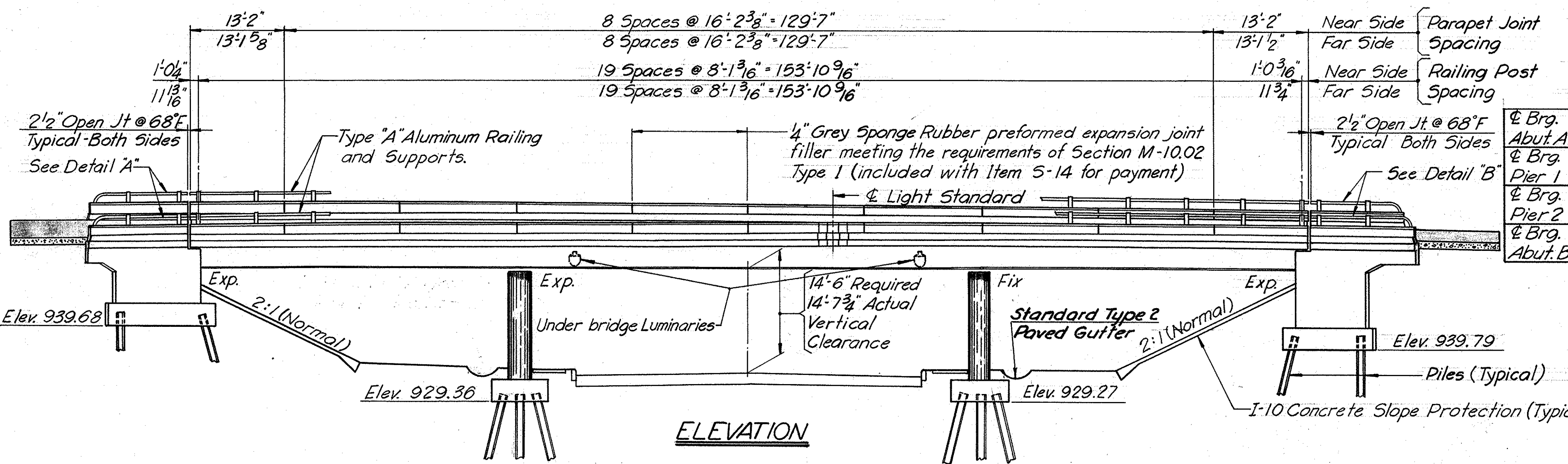
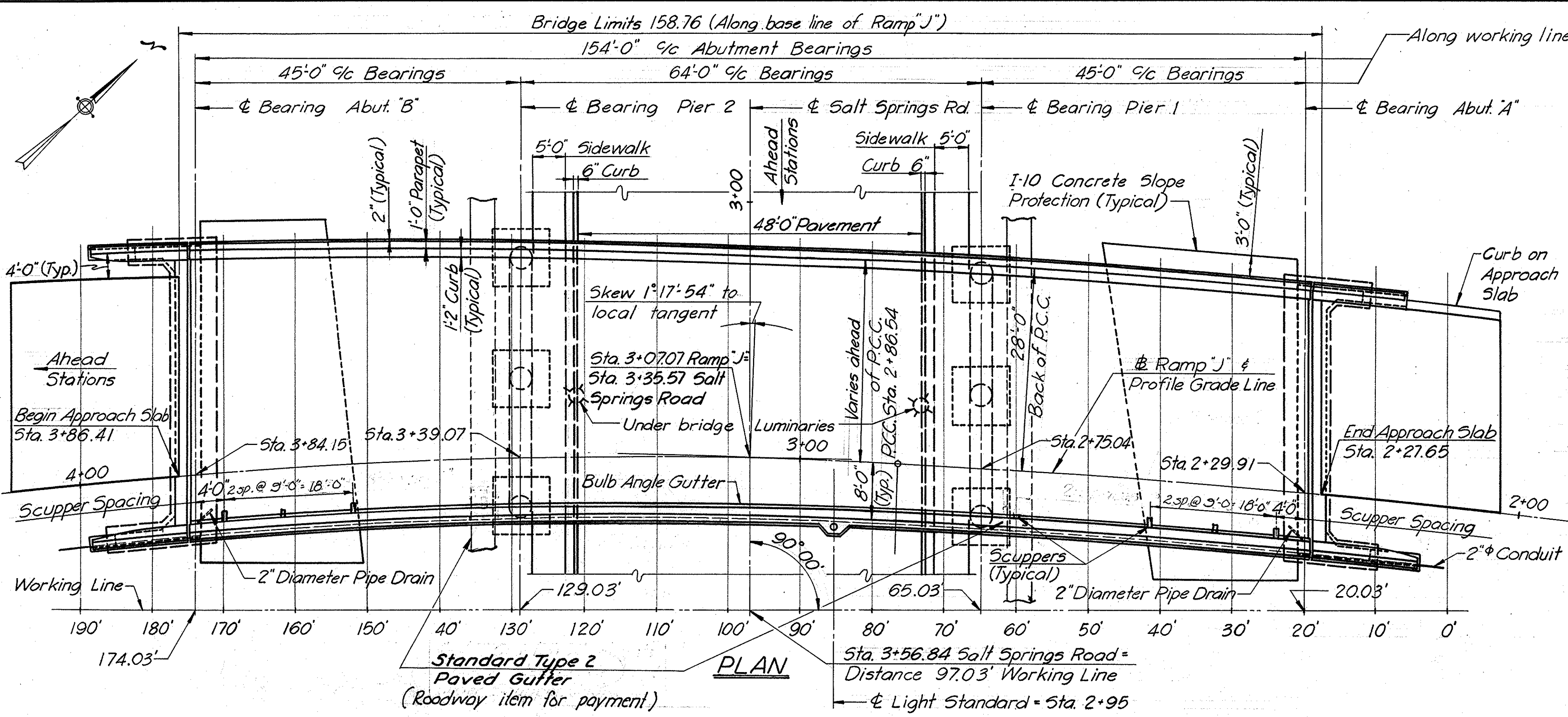
- The entire parapet structure except top and bottom surfaces of sidewalks and ramping.
- The entire surface of piers and abutments except bridge ends, back walls and the face of abutments between outside brams.

UTILITY LINES: All expense involved in relocating the affected utility lines shall be borne by the Owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

MAINTENANCE AND PROTECTION OF TRAFFIC: Two lanes of traffic with a minimum horizontal width of 28'-0" shall be maintained on Salt Springs Road at all times. The Contractor shall safeguard the traveling public by providing platforms, nets or other suitable protection above the traveled lanes. A minimum vertical clearance of 14'-0" shall be provided at all times.

OFFSETS FROM WORKING LINE & TOP OF SLAB ELEVATIONS

DISTANCE ALONG WORKING LINE	LOCATION						
	LEFT CURB		BASE LINE		RIGHT CURB		
	Offset	Elev.	Offset	Elev.	Offset	Elev.	
0'	7.65'	951.37	14.52'	951.83	41.50'	953.64	
10'	8.68'	951.45	15.54'	951.92	42.50'	953.74	
20'	9.63'	951.53	16.49'	952.00	43.43'	953.84	
30'	10.51'	951.59	17.36'	952.07	44.29'	953.93	
40'	11.32'	951.65	18.17'	952.13	45.08'	954.01	
50'	12.06'	951.70	18.91'	952.18	45.80'	954.09	
60'	12.72'	951.74	19.57'	952.23	46.46'	954.15	
70'	13.32'	951.77	20.16'	952.27	47.04'	954.21	
80'	13.84'	951.80	20.68'	952.30	47.55'	954.26	
90'	14.24'	951.81	21.08'	952.32	48.00'	954.31	
100'	14.50'	951.82	21.33'	952.33	48.38'	954.35	
110'	14.62'	951.82	21.45'	952.34	48.69'	954.40	
120'	14.59'	951.81	21.43'	952.33	48.92'	954.44	
130'	14.43'	951.79	21.26'	952.32	49.10'	954.47	
140'	14.12'	951.77	20.96'	952.30	49.20'	954.51	
150'	13.61'	951.74	20.51'	952.28	49.23'	954.54	
160'	13.08'	951.69	19.93'	952.24	49.20'	954.58	
170'	12.35'	951.64	19.21'	952.20	49.07'	954.61	
180'	11.48'	951.59	18.34'	952.15	48.80'	954.63	
190'	10.46'	951.52	17.33'	952.09	48.37'	954.64	
☉ Brg. Abut. A	20.03'	9.63'	951.53	16.49'	952.00	43.44'	953.84
☉ Brg. Pier 1	65.03'	13.03'	951.76	19.88'	952.25	46.76'	954.18
☉ Brg. Pier 2	129.03'	14.45'	951.79	21.28'	952.32	49.08'	954.47
☉ Brg. Abut. B	174.03'	12.02'	951.62	18.88'	952.22	48.98'	954.62



NOTE:
The spacing dimensions for Parapet Joints and Railing Posts are measured along inside face of Parapet.

REFERENCE shall be made to Supplemental Specification 3-307 revised 10-1-64 and Std. Dwg. SD-2-64 dated 11-25-64

DESIGN LOADING: of 2000 (S7)

CONCRETE CLASS "C": Basic unit stress 1,333 p.s.i.

CONCRETE CLASS "E": Basic unit stress 1,133 p.s.i.

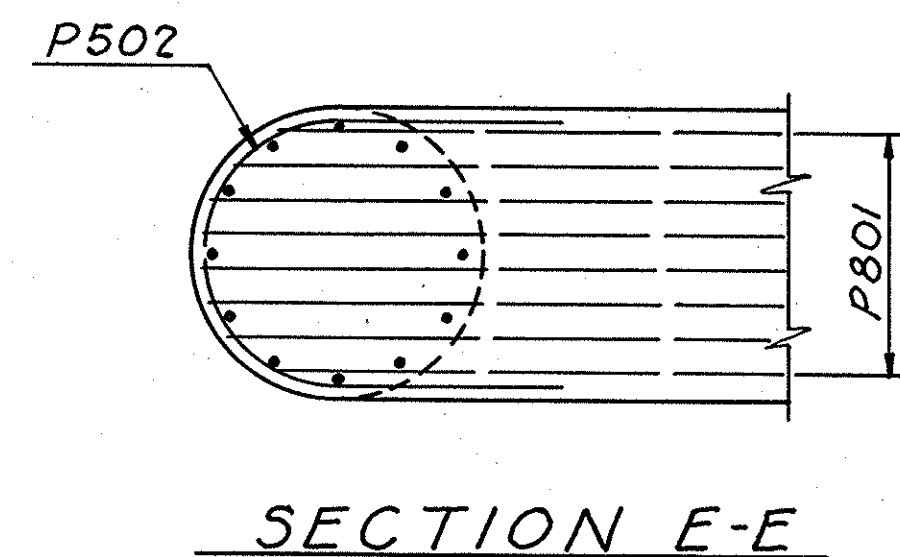
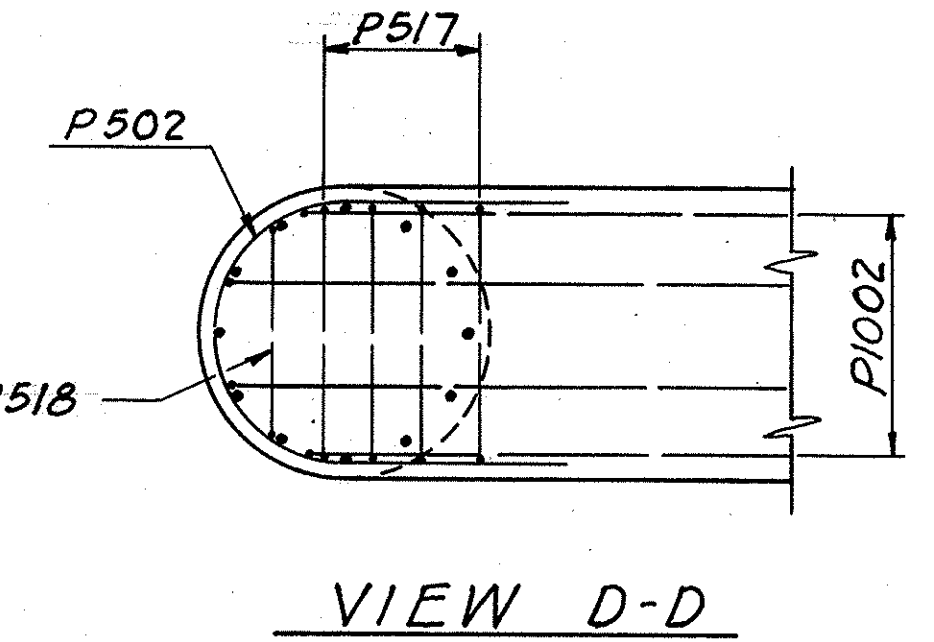
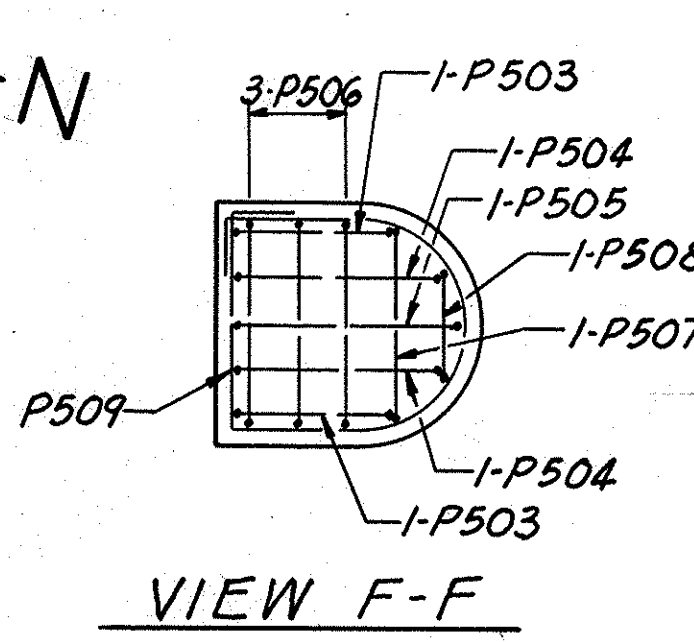
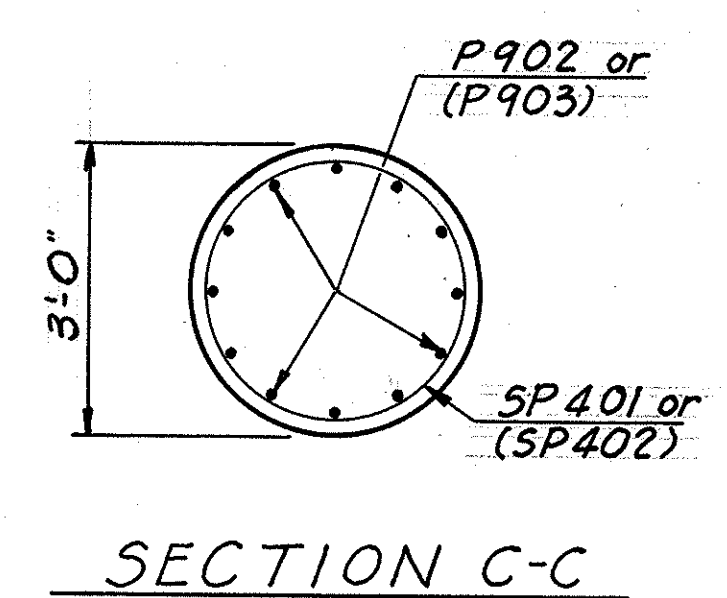
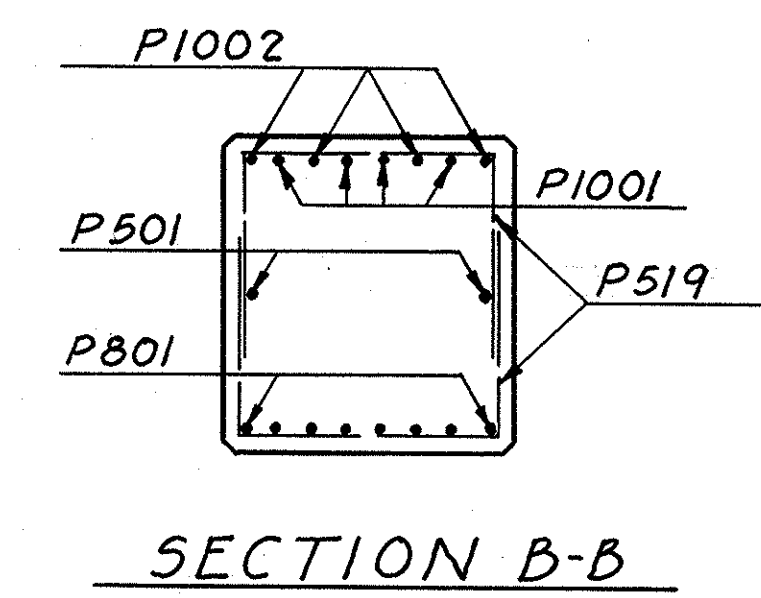
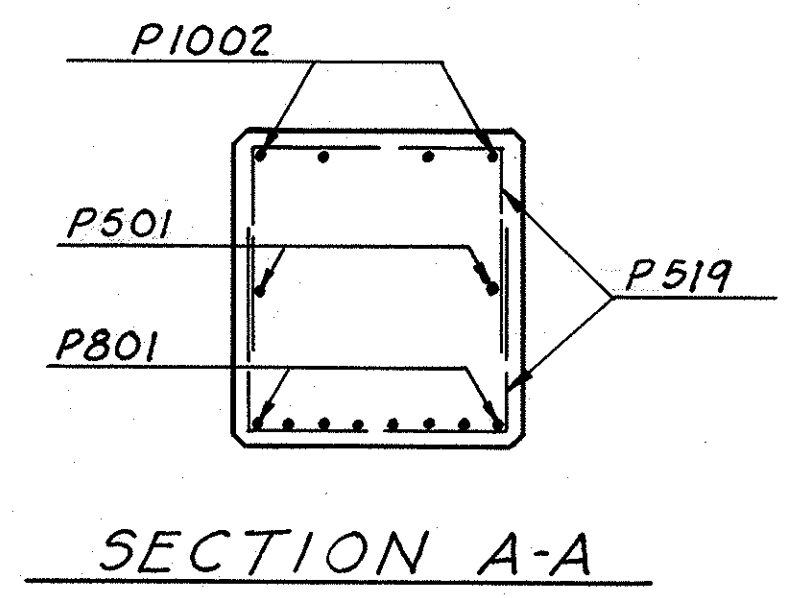
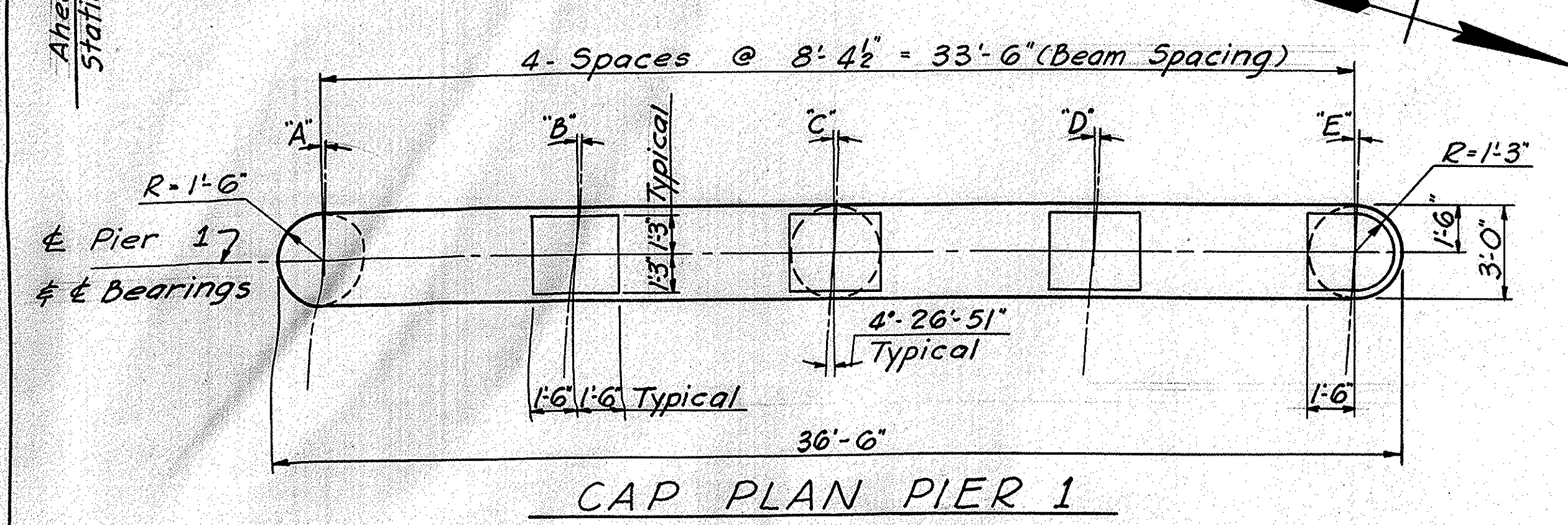
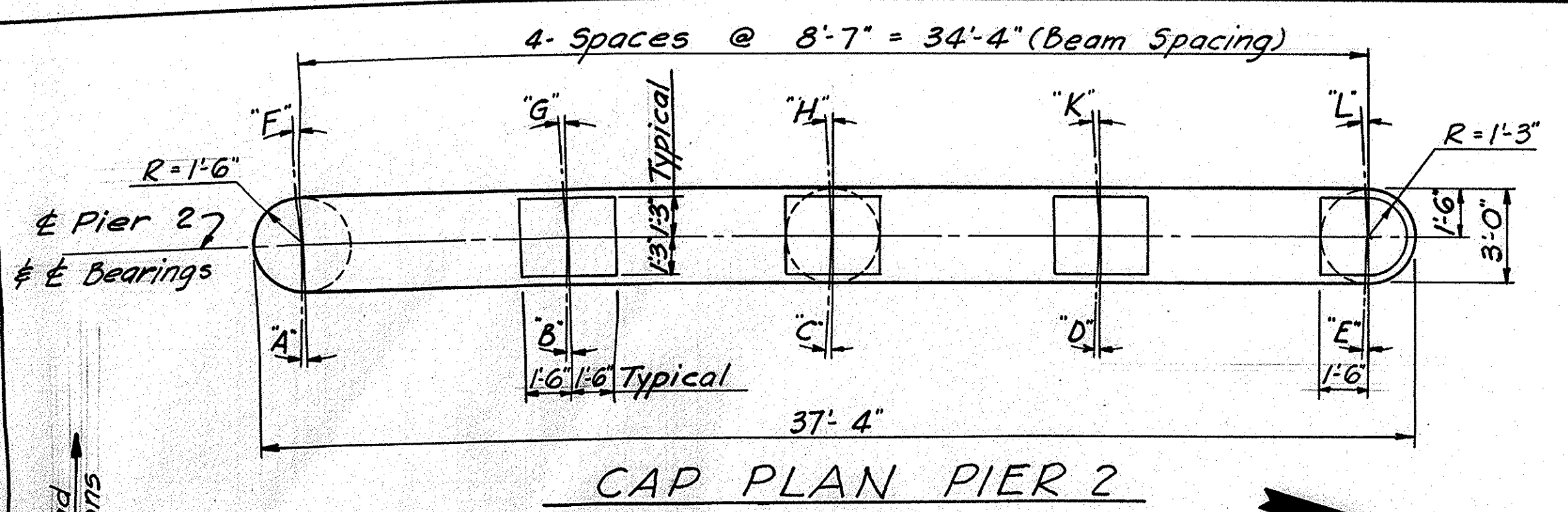
STRUCTURAL STEEL: ASTM A36, basic unit stress 20,000 p.s.i. (ASTM A7 and A373 steel not permitted.)

REINFORCING STEEL: ASTM A15, A16 and A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 p.s.i. Except spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 p.s.i.

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ROCHESTER, PENNSYLVANIA

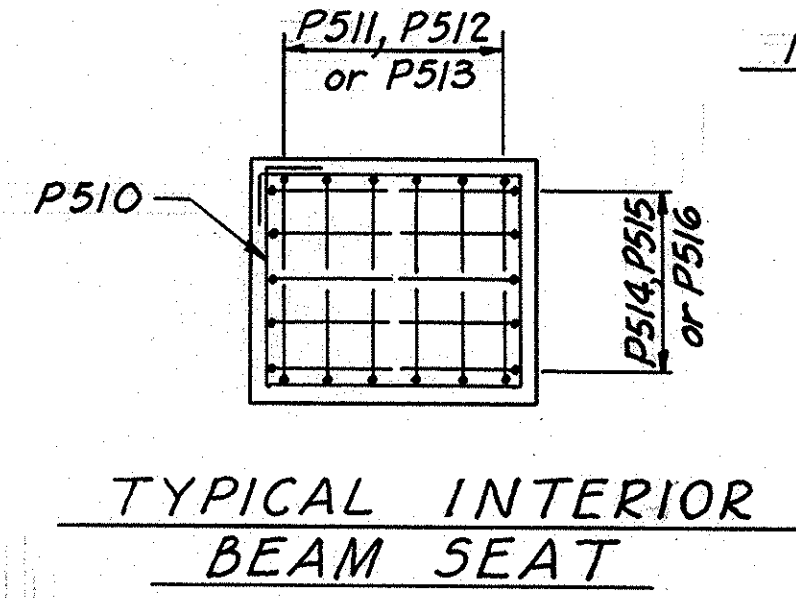
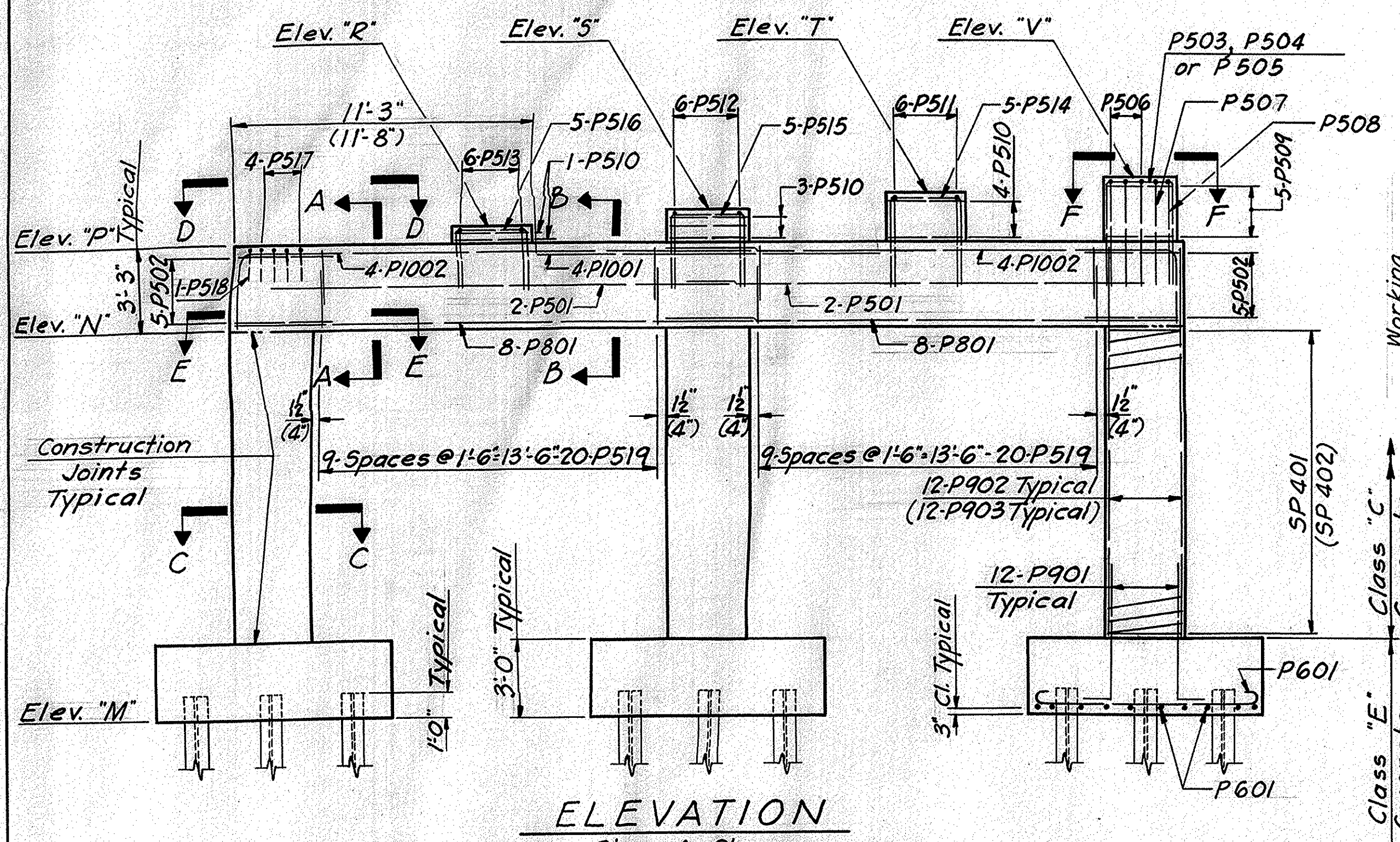
GENERAL PLAN & ELEVATION
BRIDGE NO. MAH-18-RAMP J
OVER SALT SPRINGS ROAD

MAHONING COUNTY		STA. 3+07.07	
Designed	Drawn	Traced	Checked
E.F.R.	E.R.J.	E.R.J.	R.F.L.
			Reviewed Date
			8-26-63
			Revised

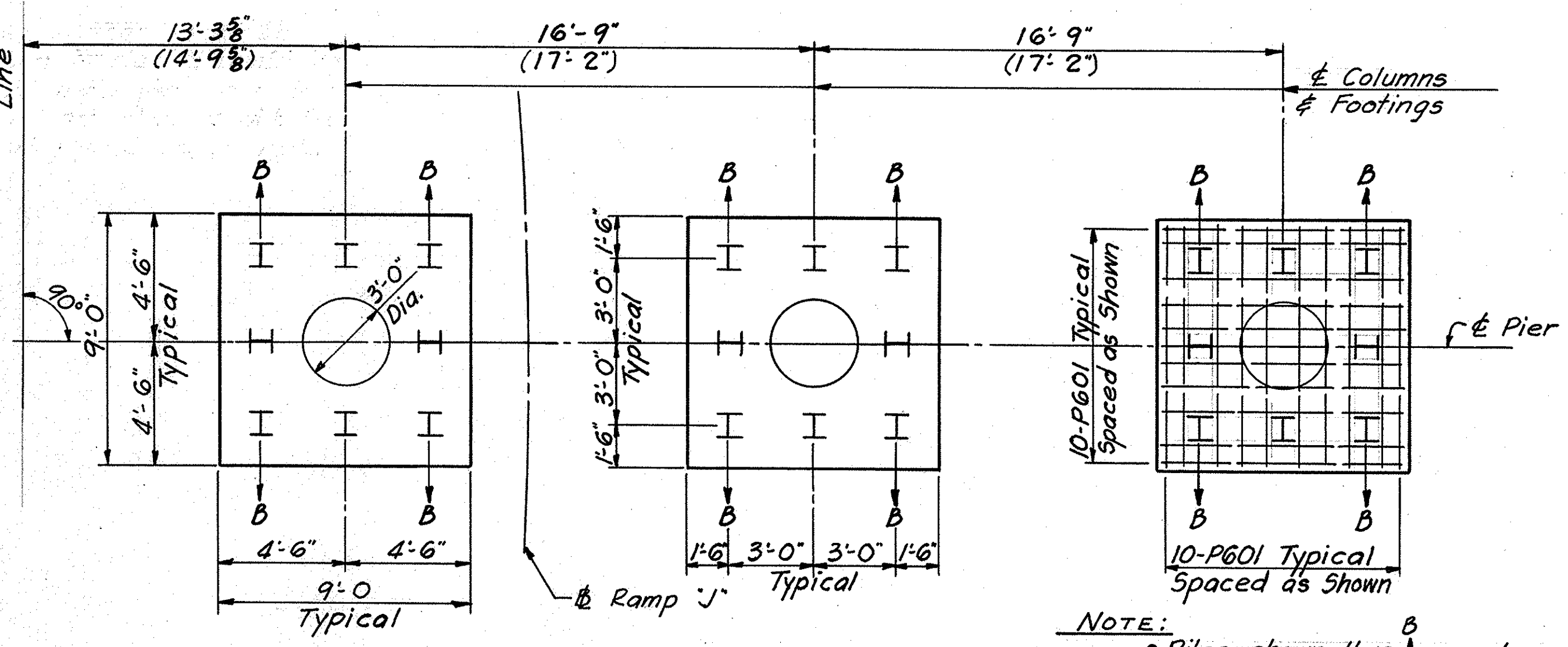


NOTES:

- Super-Structure Grounds: A No 1/0 AWG 7 strand soft annealed bare copper cable shall be encased in the outside column of pier No. 1. Connect (by Exothermic weld process) lower end of cable to a steel pile. Extend cable (in one continuous length) through top of pier with lead of sufficient length to exothermic weld upper end of cable to outside beam of super-structure.
- Payment for electrical grounds is included in the lump sum bid for 5-25 "Electrical Lighting System Complete."



NOTE:
• Special care shall be taken in placing reinforcing steel in the pier cap so that it will not interfere with the drilling of anchor rod holes.

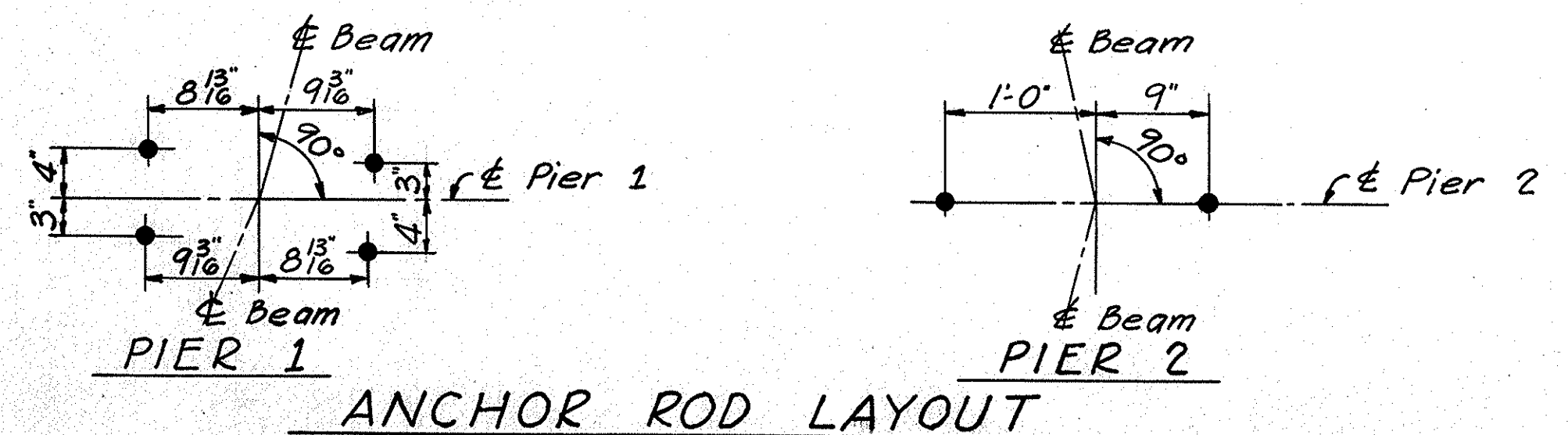


BEAM ANGLES

A	1°-20'-33"
B	1°-31'-44"
C	1°-42'-55"
D	1°-54'-06"
E	2°-05'-17"
F	3°-33'-38"
G	2°-42'-51"
H	1°-52'-00"
K	1°-01'-07"
L	0°-10'-11"

PIER ELEVATIONS

Elevation	Pier 1	Pier 2
M	929.27	929.36
N	944.05	944.39
P	947.30	947.64
R	947.90	948.30
S	948.50	948.96
T	949.10	949.62
V	949.71	950.29



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PIERS
BRIDGE NO. MAH-18-RAMP J
OVER SALT SPRINGS ROAD

MAHONING COUNTY	STA. 3+07.07					
Designed	Drawn	Troced	Checked	Reviewed	Date	Revised
R.C.	R.C.	R.L.S.			8-26-63	

**MAHONING COUNTY
MAH-18-15-50**

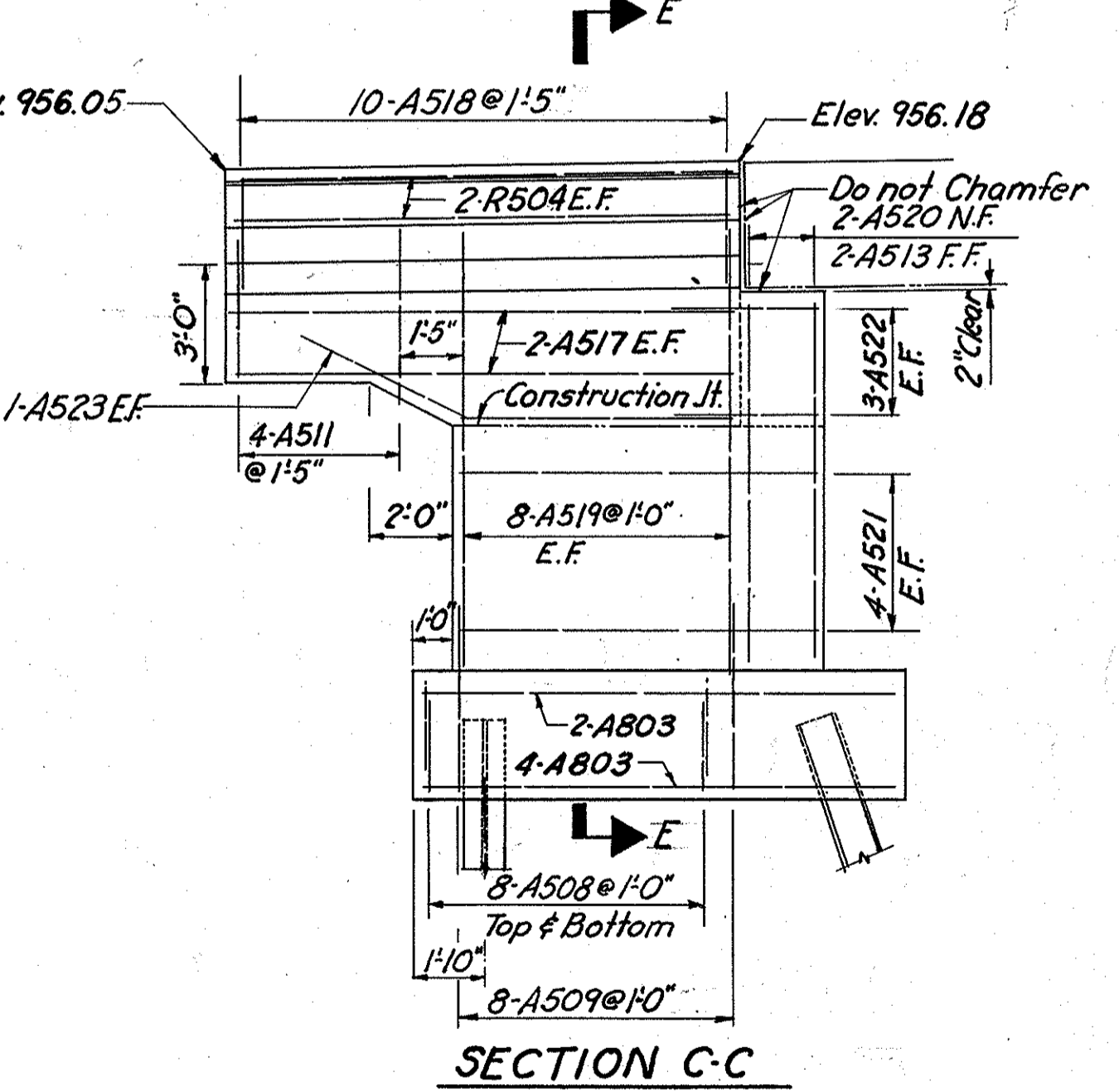
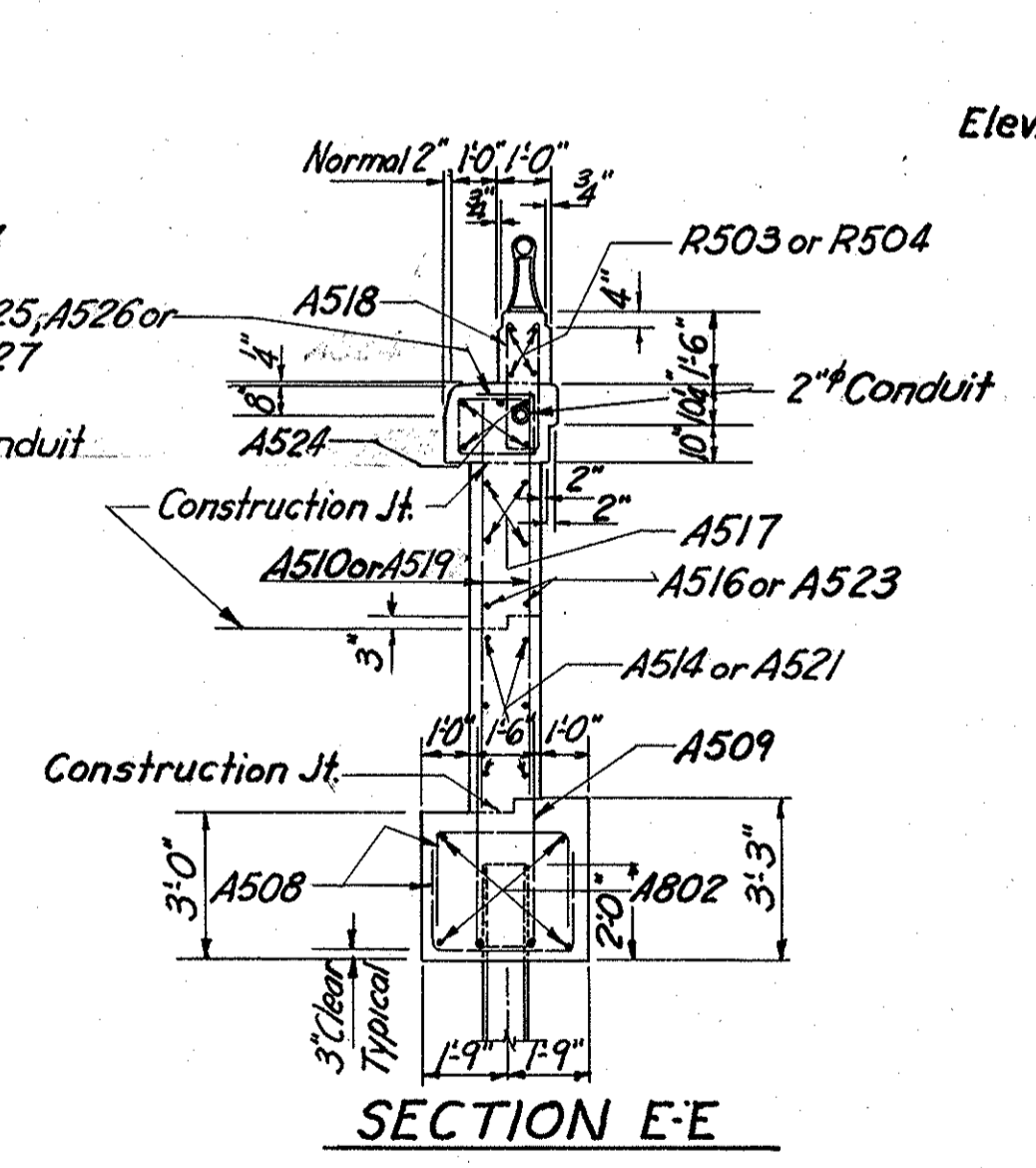
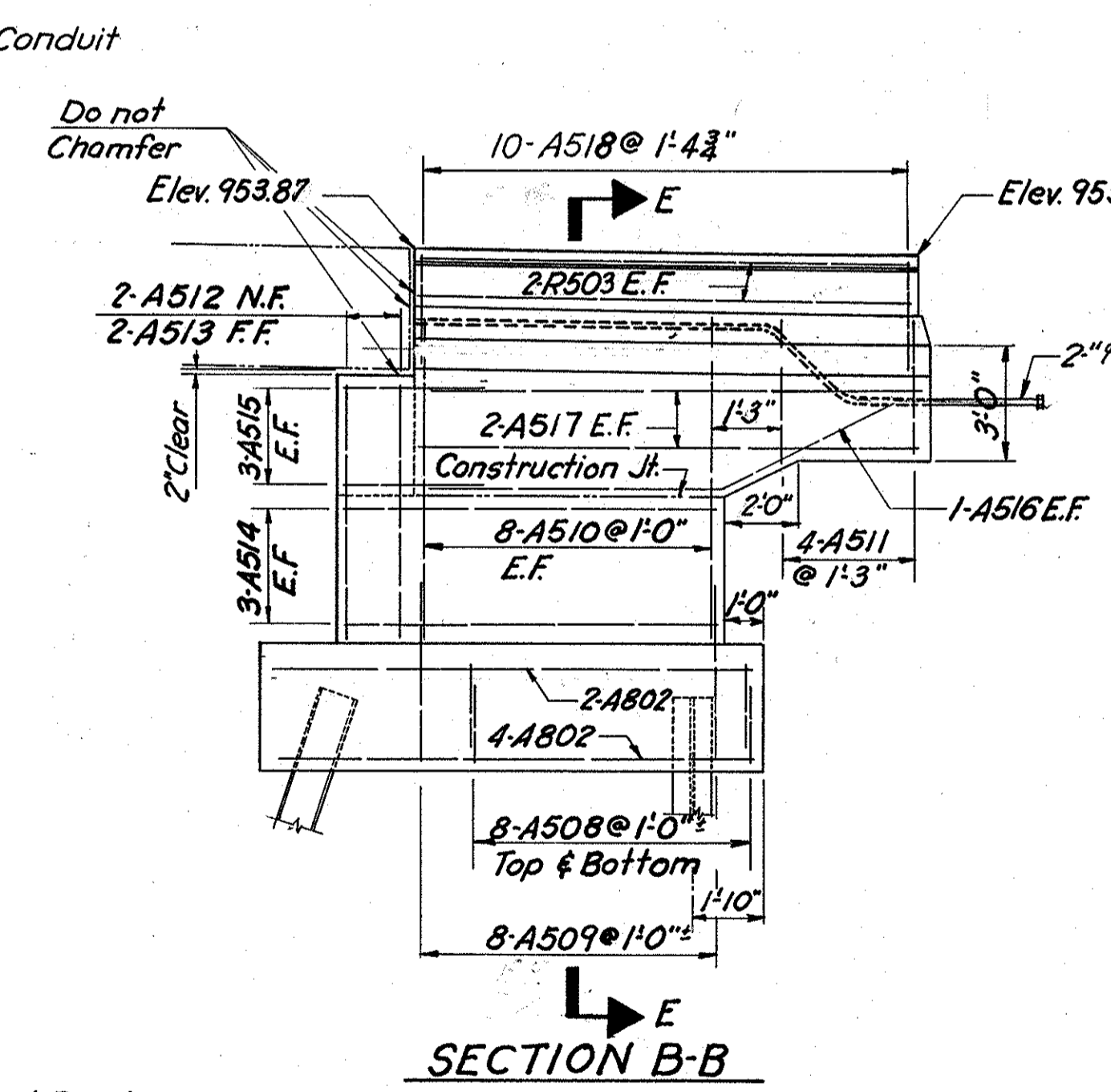
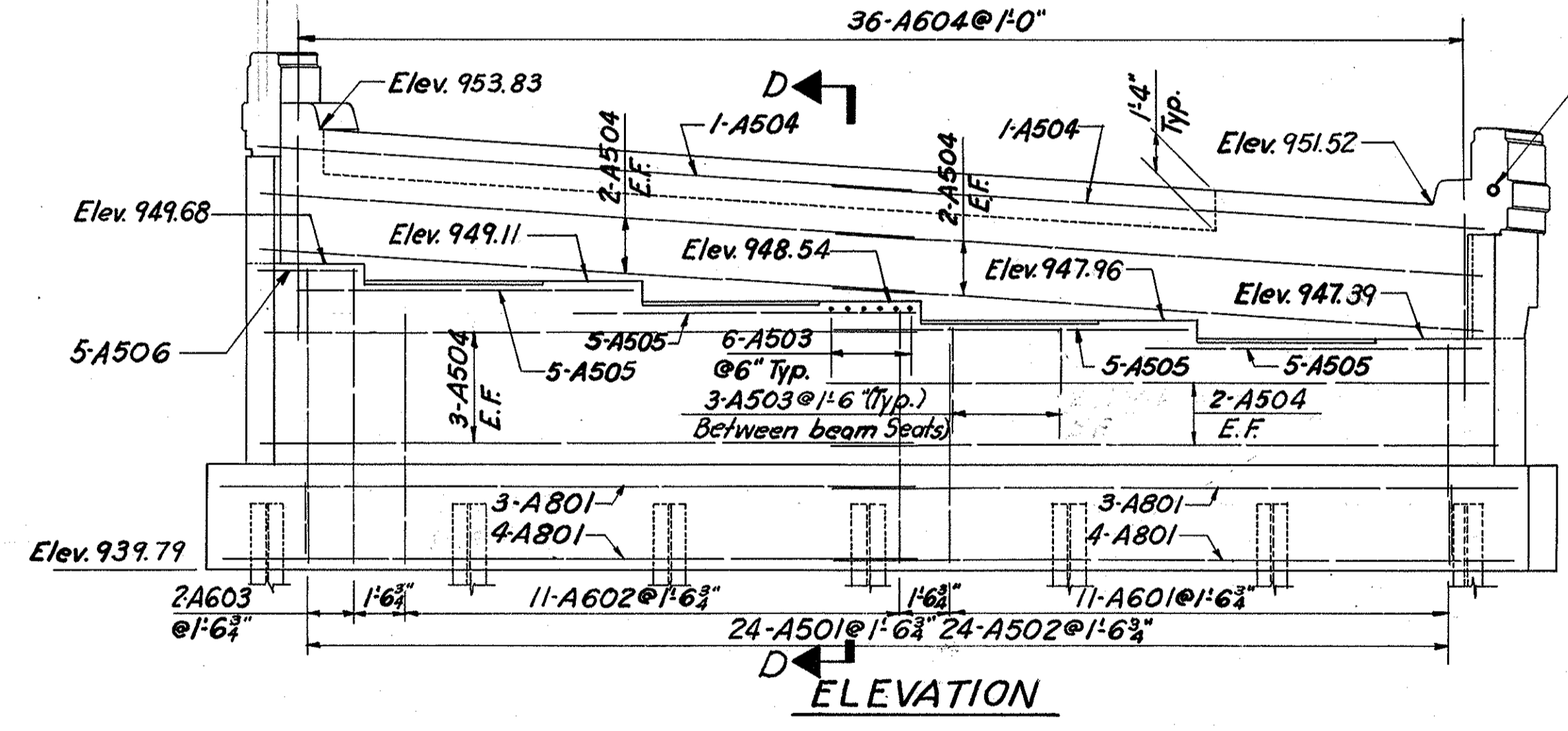
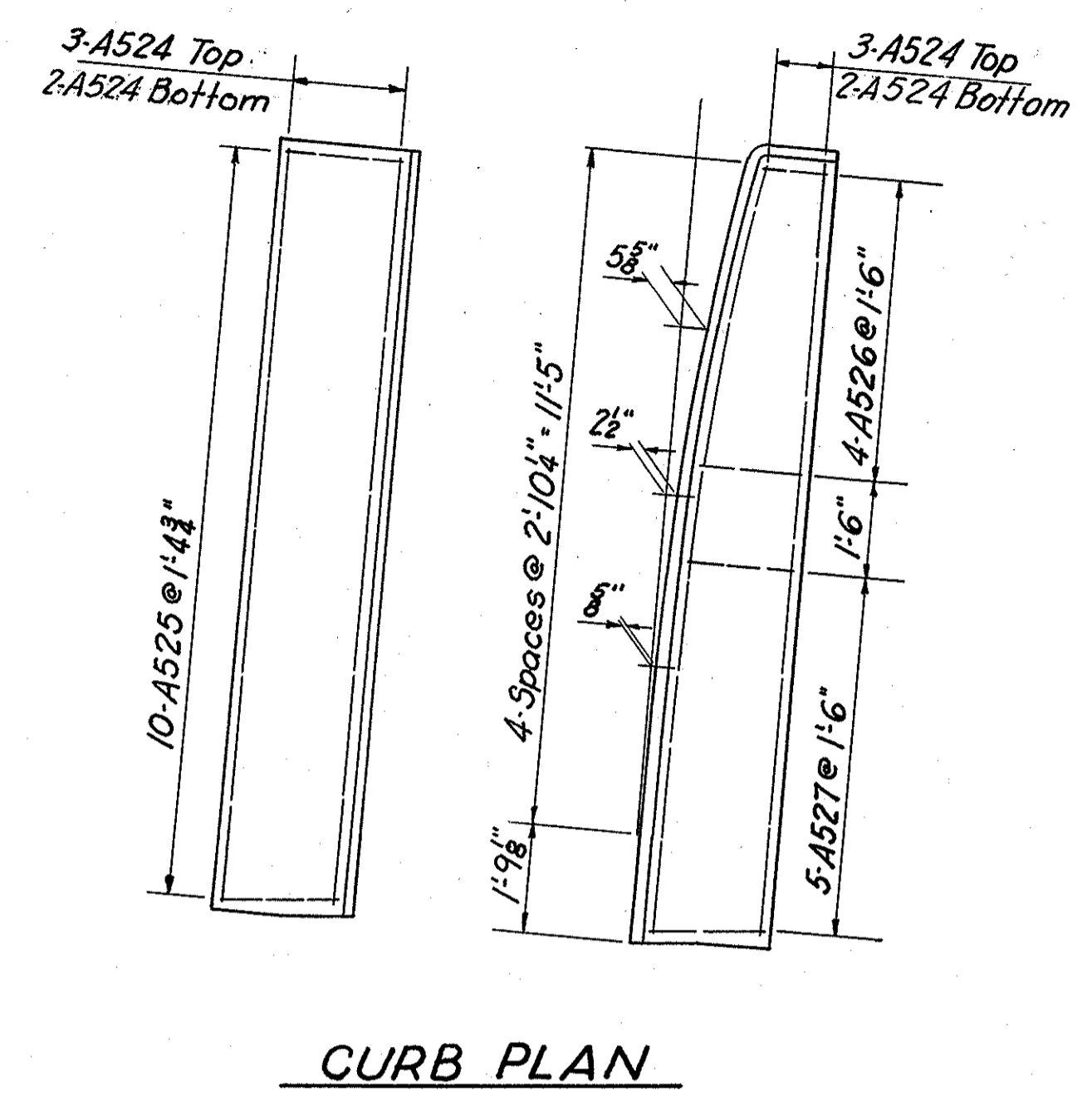
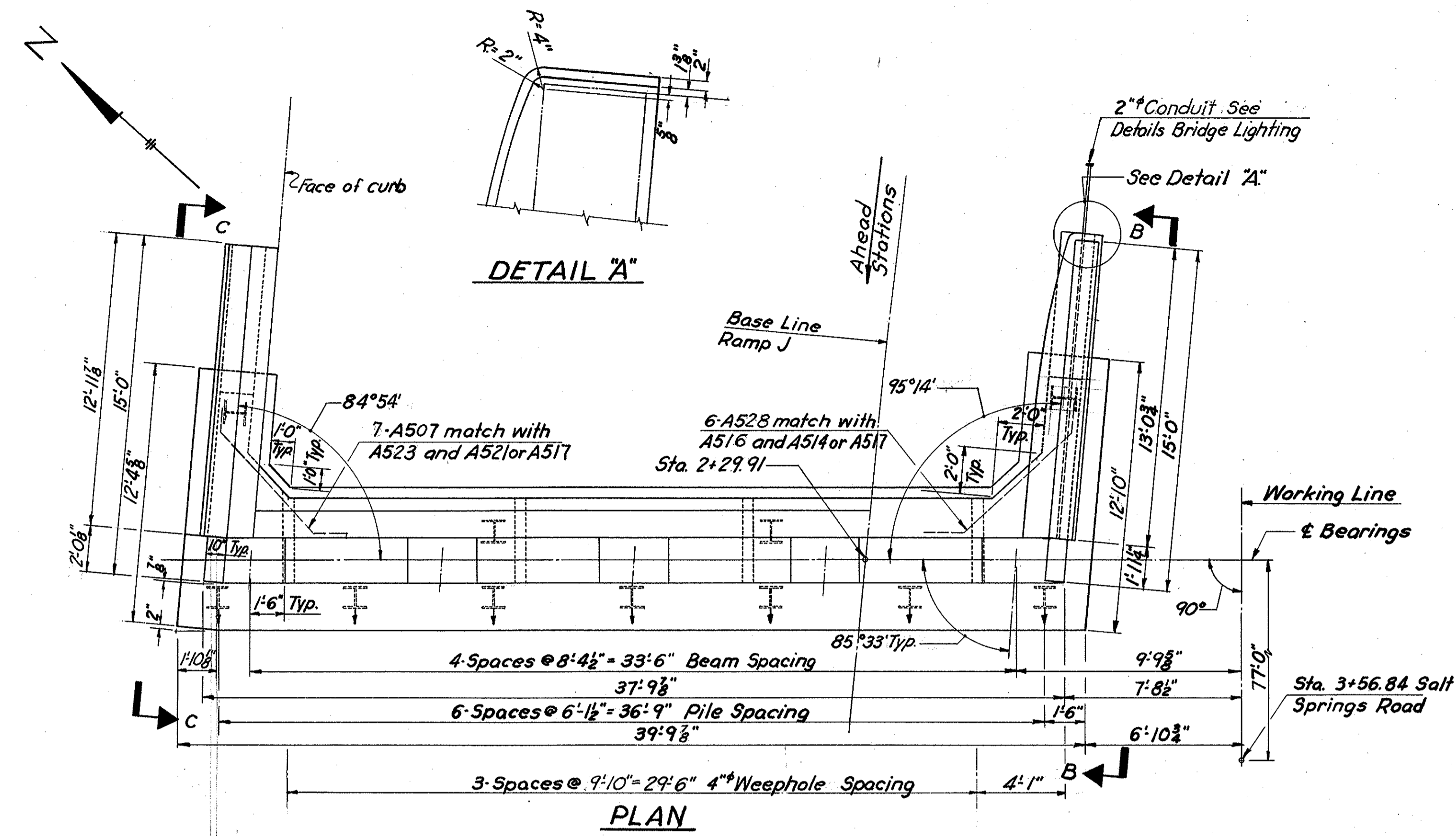
NOTES:
 Porous backfill 2 feet thick full length of abutment shall extend up to the underside of the approach slab and outward to the wings. Excavation there for, in excess of that required for construction of the abutments, shall be considered as paid for in the bid price per Cu.Yd. paid for porous backfill.

All abutment concrete shall be Class "E".
 All parapet concrete shall be Class "C".
 Concrete and reinforcing steel above parapet construction joint shall be included with railing S-14 for payment.

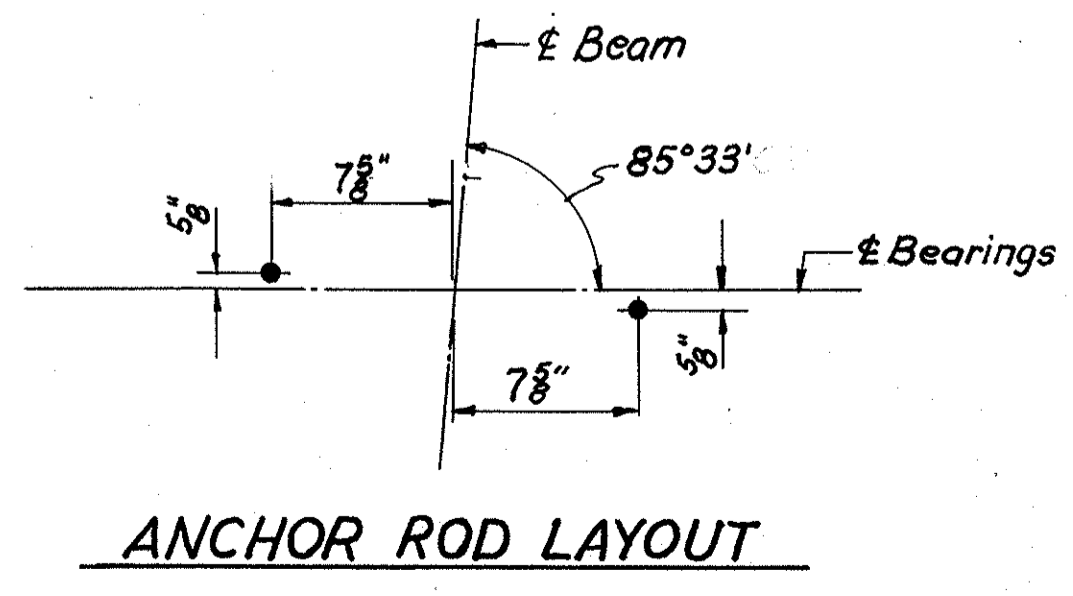
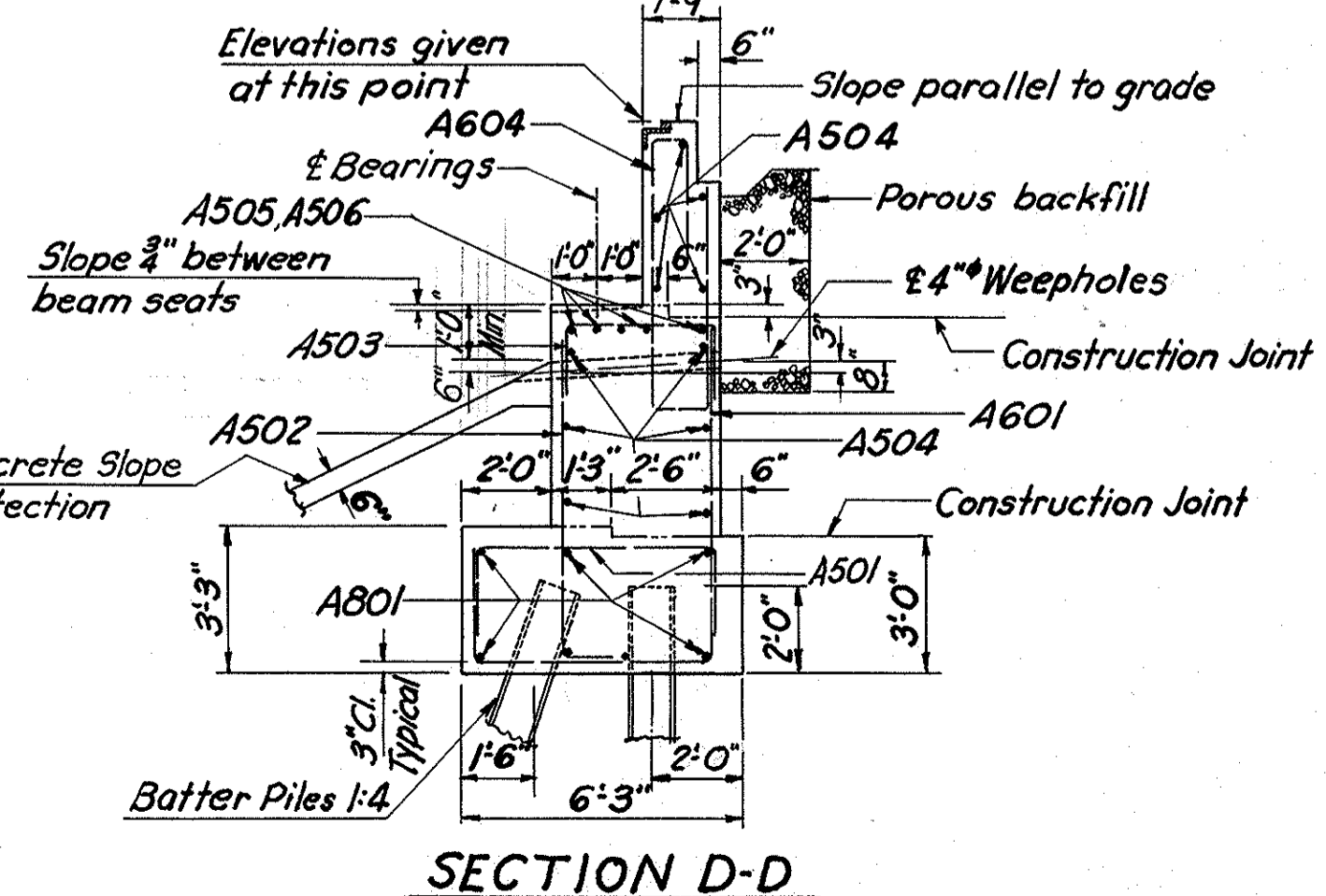
All abutment parapet railings shall be continuous through railing posts.
 Refer to Standard Drawing AR-1-57 (Revised 4-2-62) for details of Type "A" railing and concrete parapet.

PROCEDURE: The embankment shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade, after which the excavation shall be made for the abutments and piles driven.

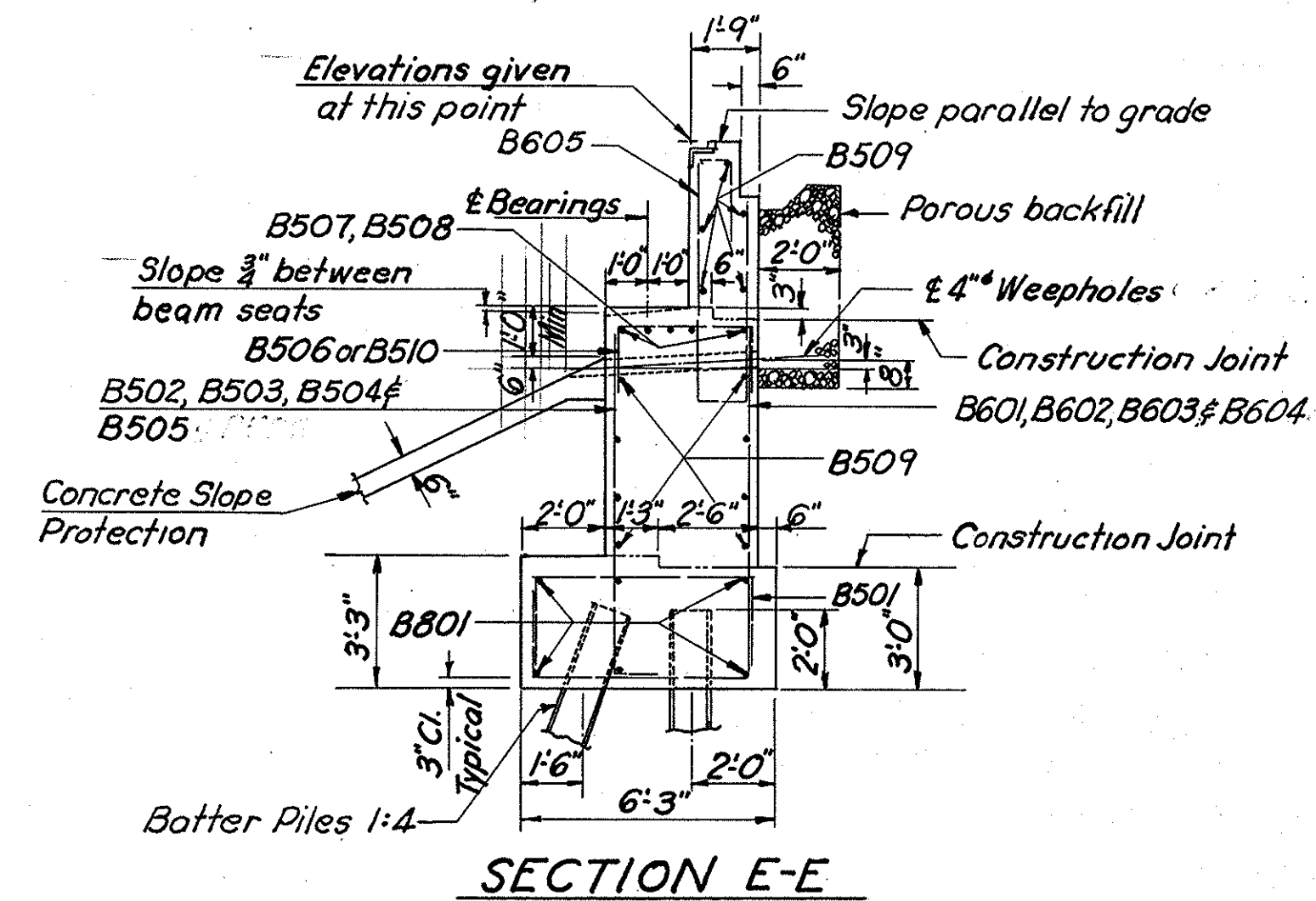
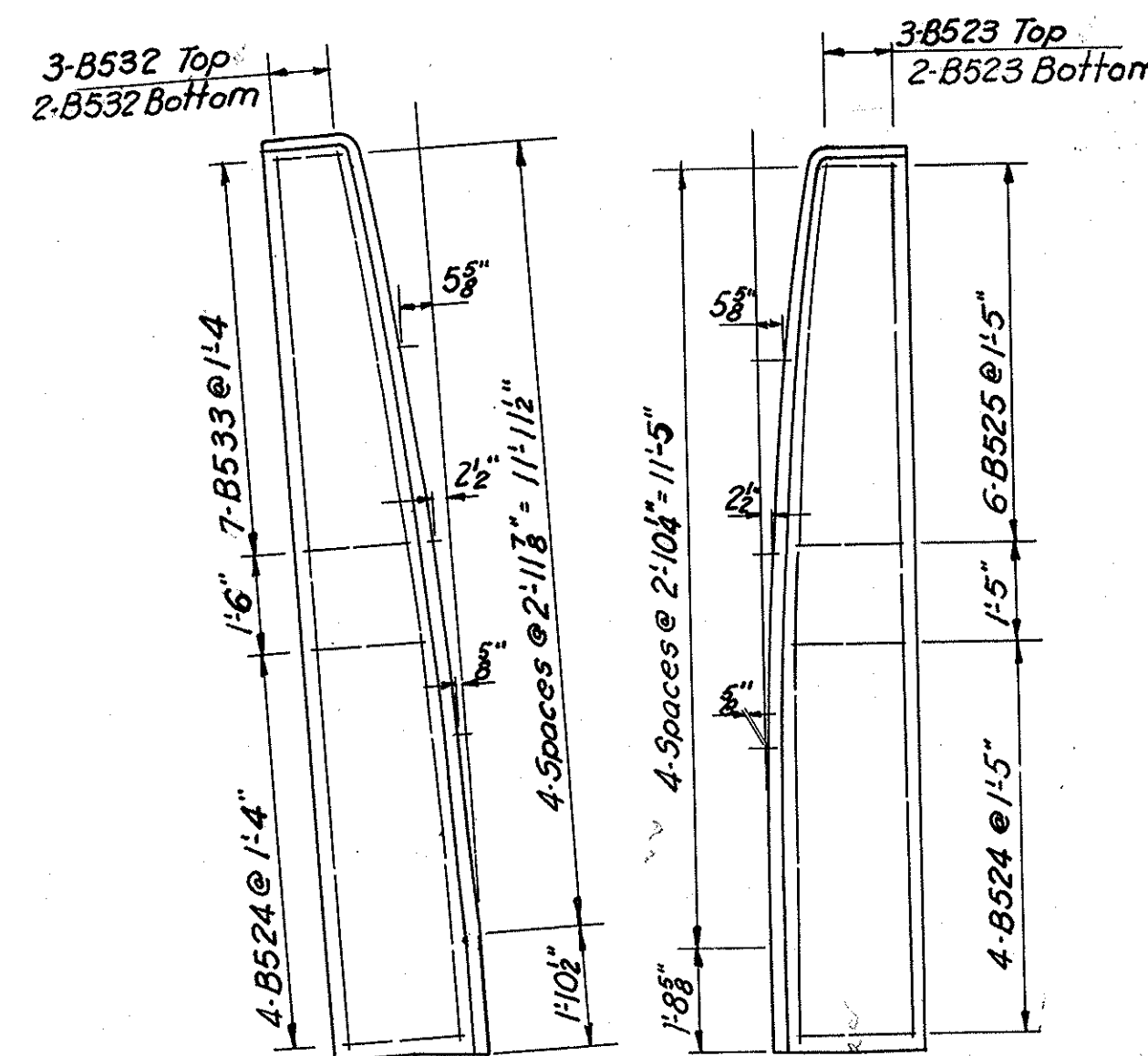
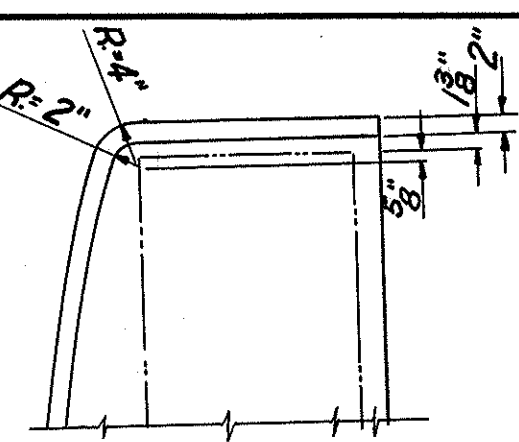
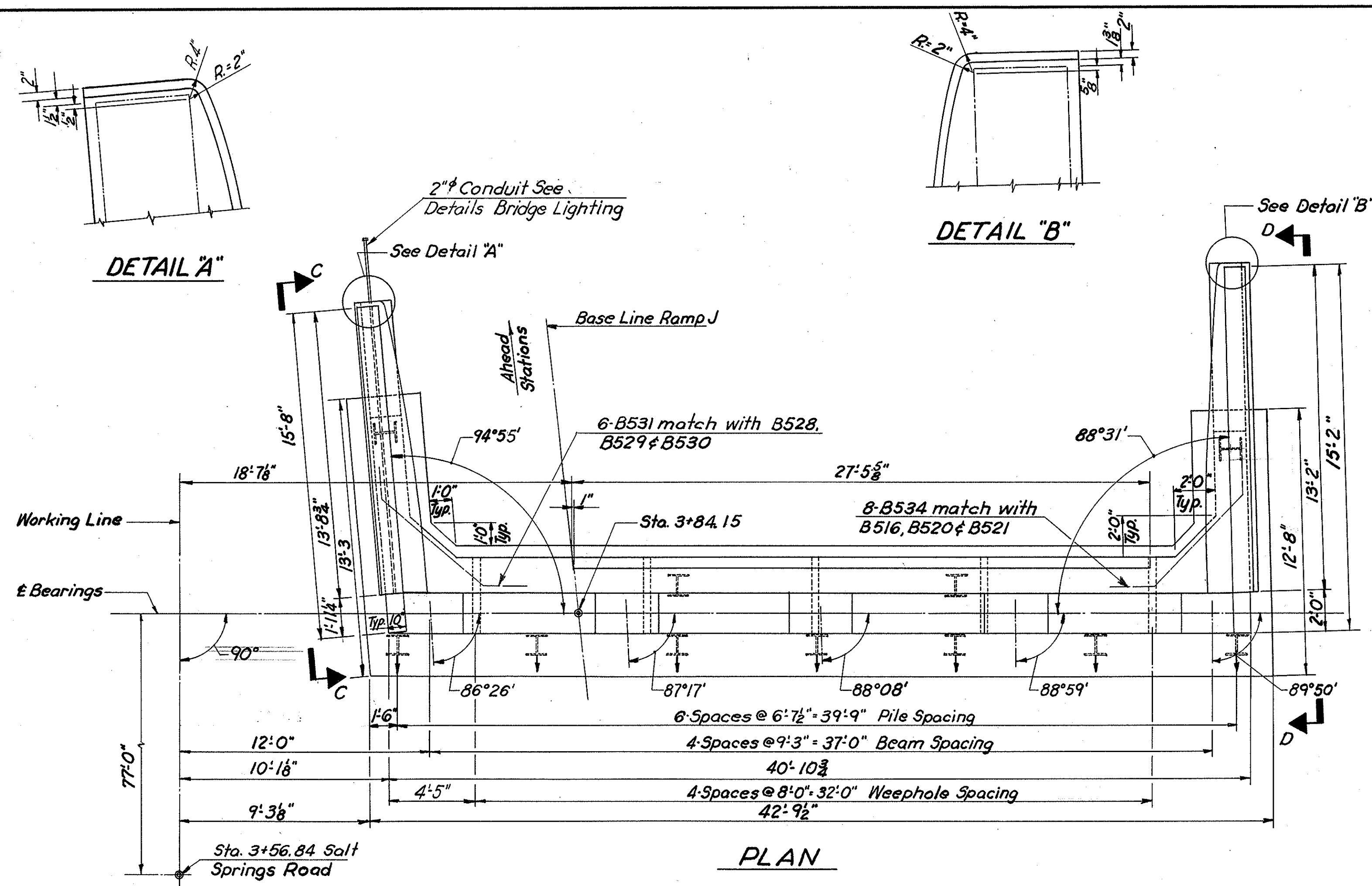
BRIDGE SEAT REINFORCING: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.



LEGEND
 N.F. = Near Face
 F.F. = Far Face
 E.F. = Each Face

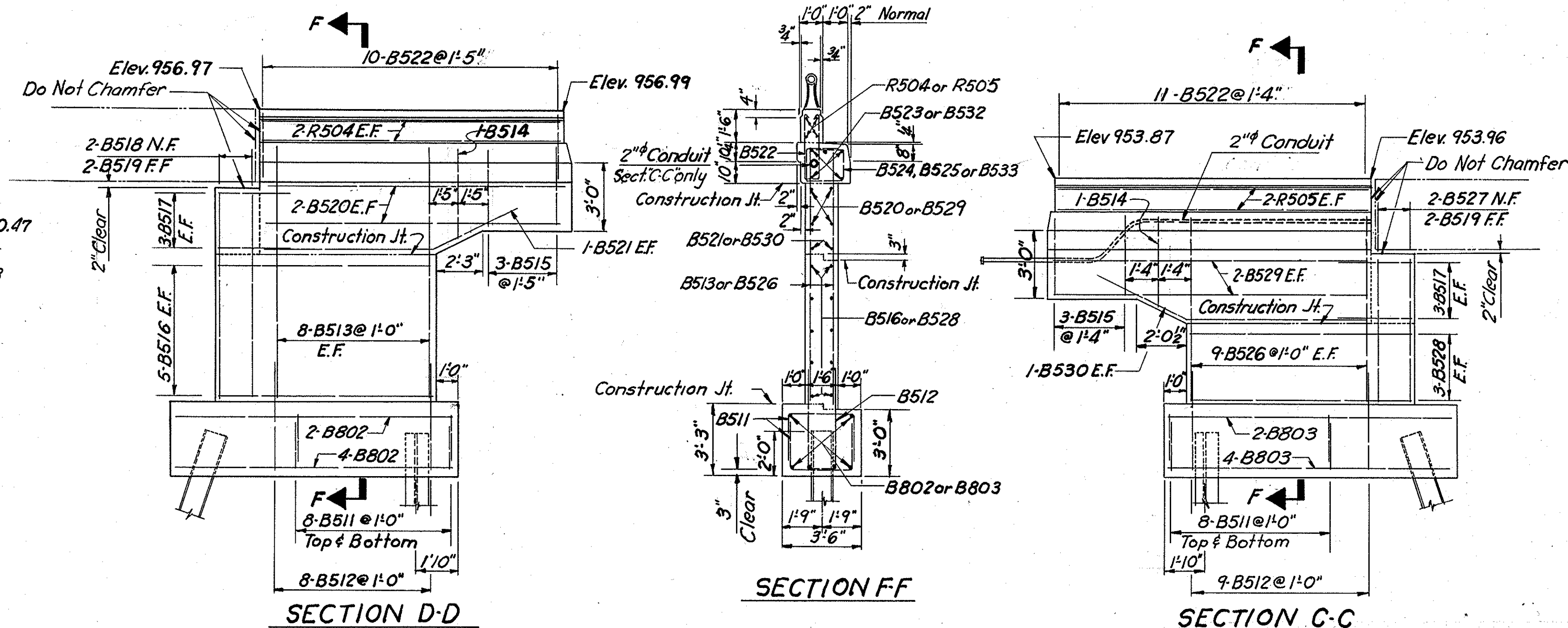
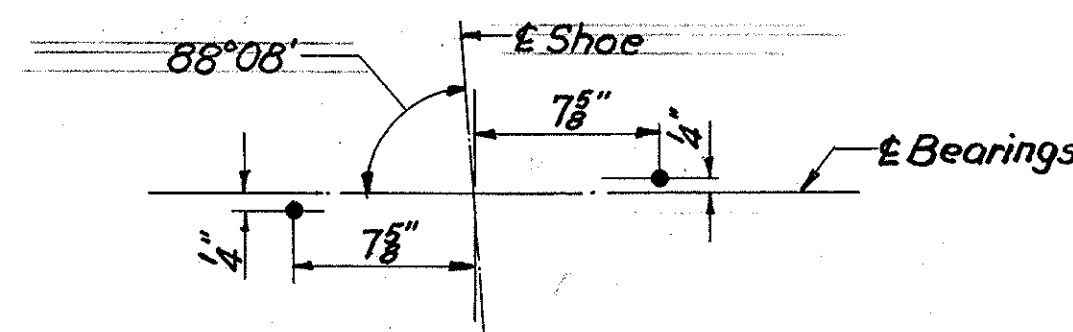
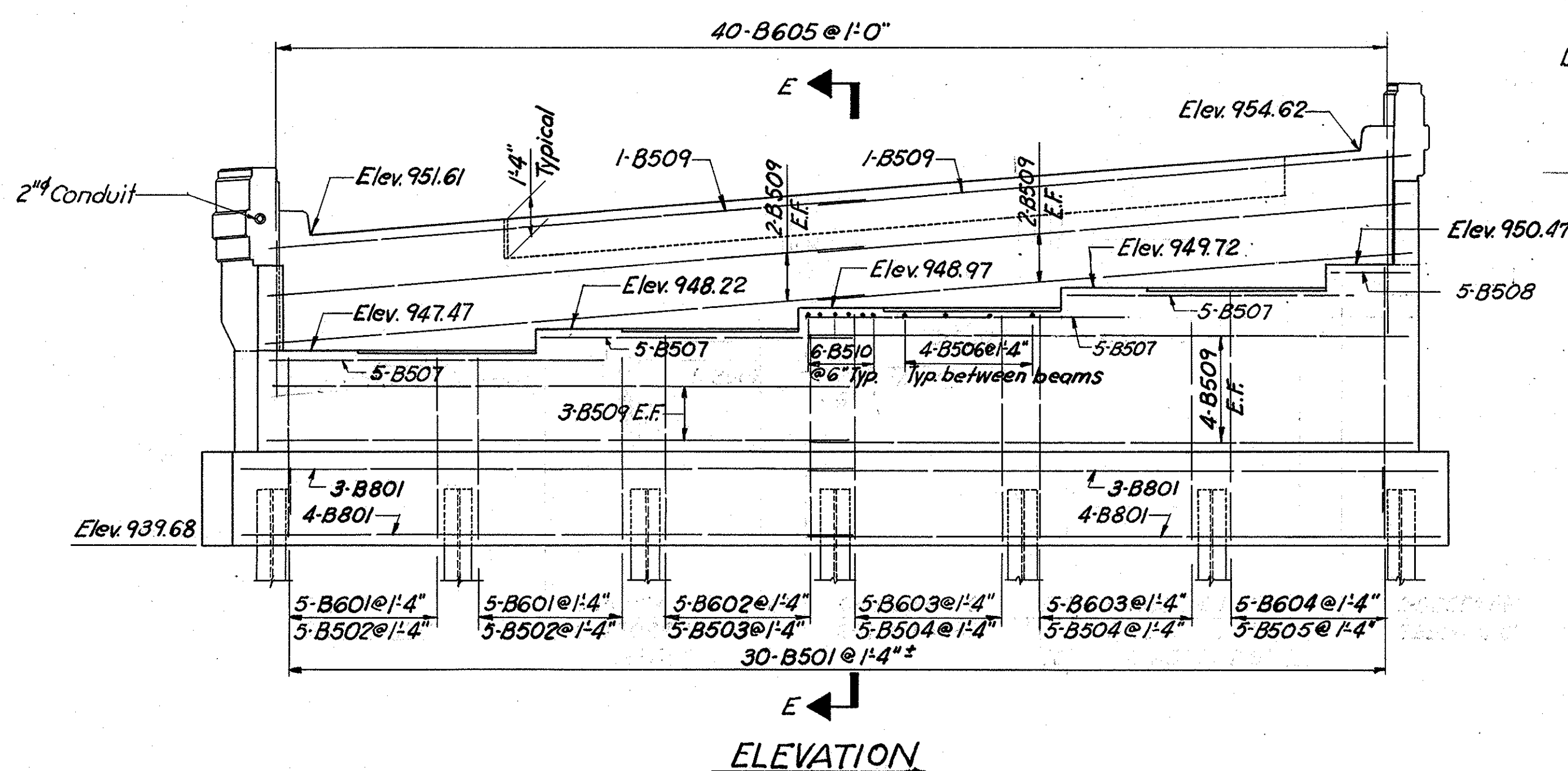


MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT A					
BRIDGE NO. MAH-18-RAMP J					
OVER SALT SPRINGS ROAD					
MAHONING COUNTY			STA. 3+07.07		
Designed	Drawn	Traced	Checked	Reviewed Date	Revised
EFR	EFR	R	A. L.	8-26-63	



NOTE: See Abutment "A" sheet for notes.

LEGEND
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face



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ROCHESTER, PENNSYLVANIA

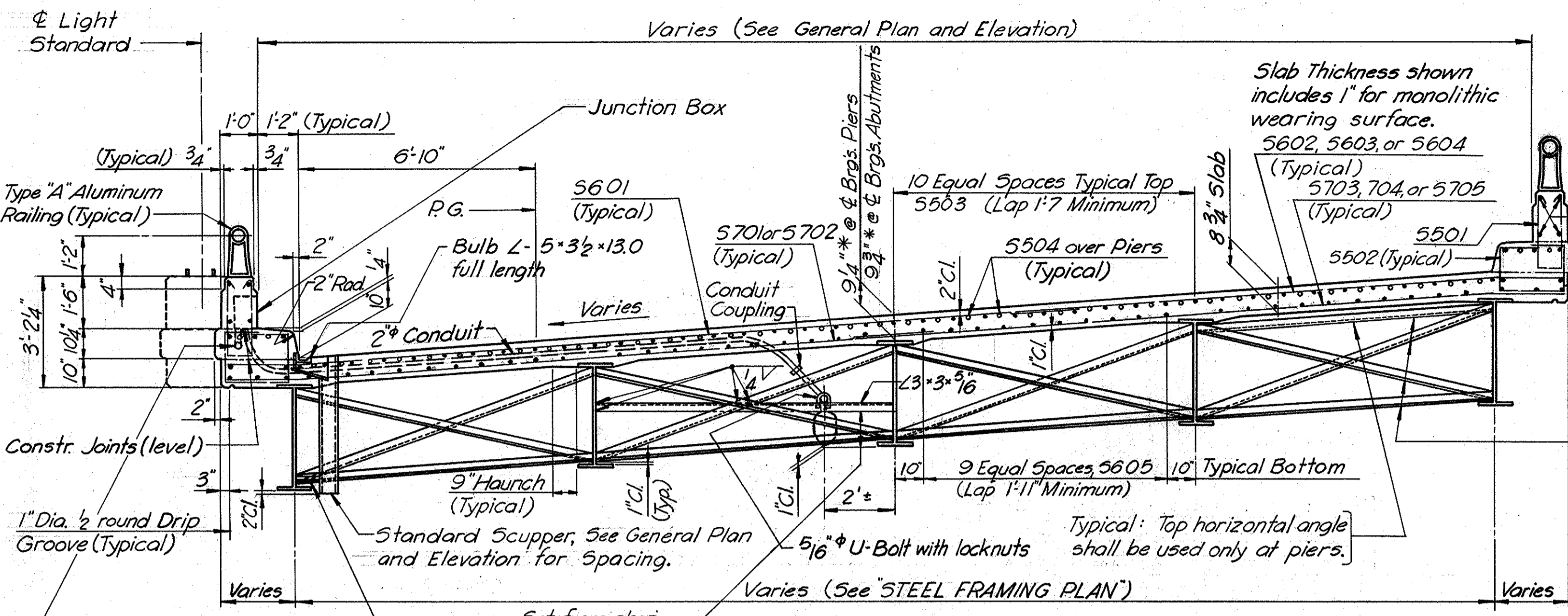
ABUTMENT B
BRIDGE NO. MAH-18-RAMP J
OVER SALT SPRINGS ROAD

MAHONING COUNTY				STA. 3+07.07	
Designed	Drawn	Traced	Checked	Reviewed	Date
EFR	EFR	B	Fl, Flc	JK	8-26-63

**MAHONING COUNTY
MAH-18-15.50**

NOTES:

- Refer to Standard Drawing AR-1-57 for details of Type 'A' aluminum railing and concrete parapet; to Standard Drawing CSB-2-56, Sheets 2 and 3 of 6 for details of expansion dams, end crossframes, 2" pipe drains, curb plates and scuppers; and for bearings refer to Standard Drawing F5B-1-62 and to Pier Sheet No. 241 and Abutment Sheets No. 242 and 243 for anchor rod layouts.
- Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
- See General Plan and Elevation for location of scuppers, 2" pipe drains and railing posts.
- CONCRETE DECK PLACING:** In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress up grade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
- DECK SLAB HAUNCH:** The haunch in the super-elevated deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" on the low side and between 9" and 12" on the high side. Except on the high side, the maximum slope shall not exceed 3" per foot. Payment for deck slab concrete shall be based on 9" width.
- All concrete shall be Class 'C'.
- ELECTRICAL LIGHTING SYSTEM:** for additional details refer to drawings 'Details Bridge Lighting and General Lighting Notes' Sheets No. 199, 201, 203 and 205.
- Luminaire support angles and 1/2" U-bolts with locknuts to be included with Structural Steel for payment.



DEFLECTION AND CAMBER

LOCATION	SPAN 1	SPAN 2	SPAN 3
Deflection due to weight of steel	0"	1/8"	0"
Deflection due to weight of remaining DL	1/8"	3/8"	1/8"
Convexity required for V.C.	1/8"	-1/8"	-1/8"
Sum of deflection and convexity	1/8"	5/16"	0"
Required Camber	0"	0"	0"

Intermediate crossframe angles 3x3x5/16
Weld both sides of vertical leg and top side of horizontal leg to beam with 1/4" continuous fillet weld.

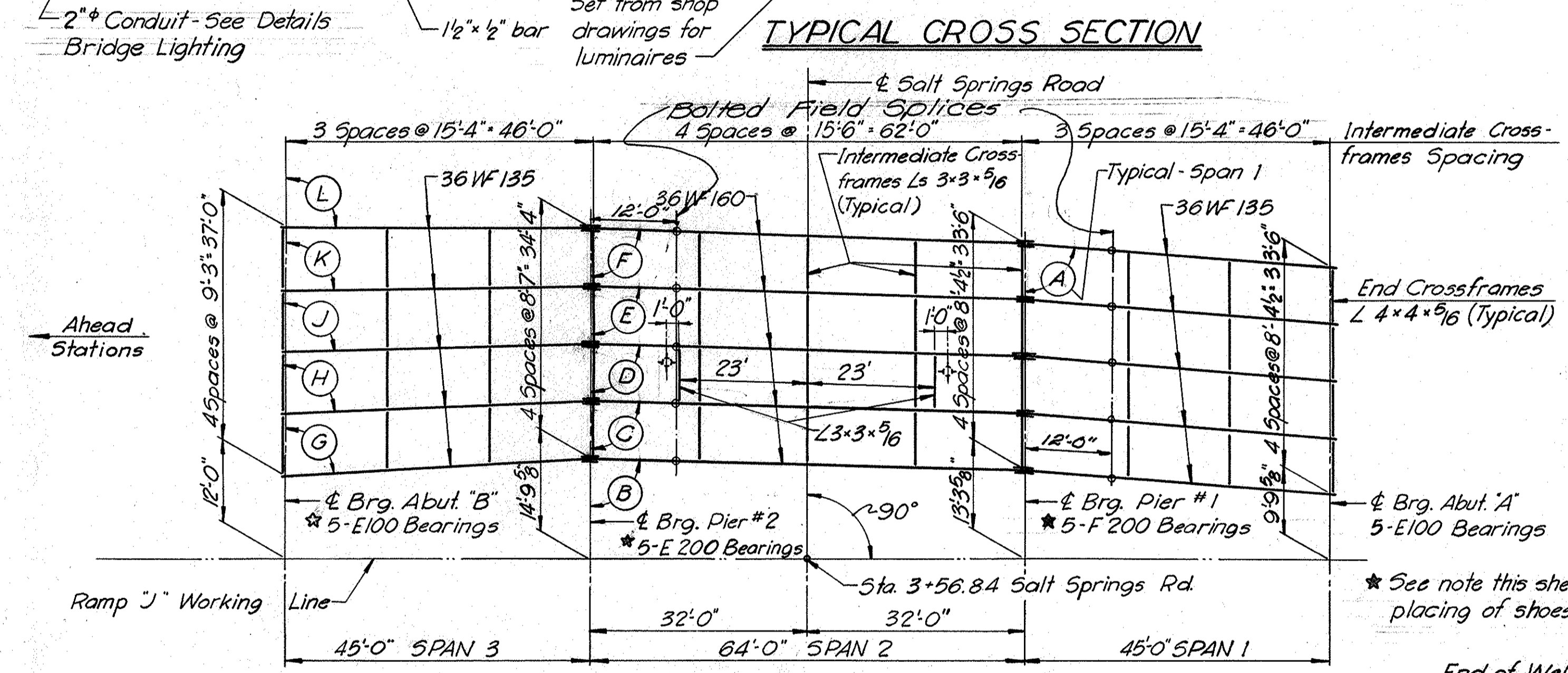
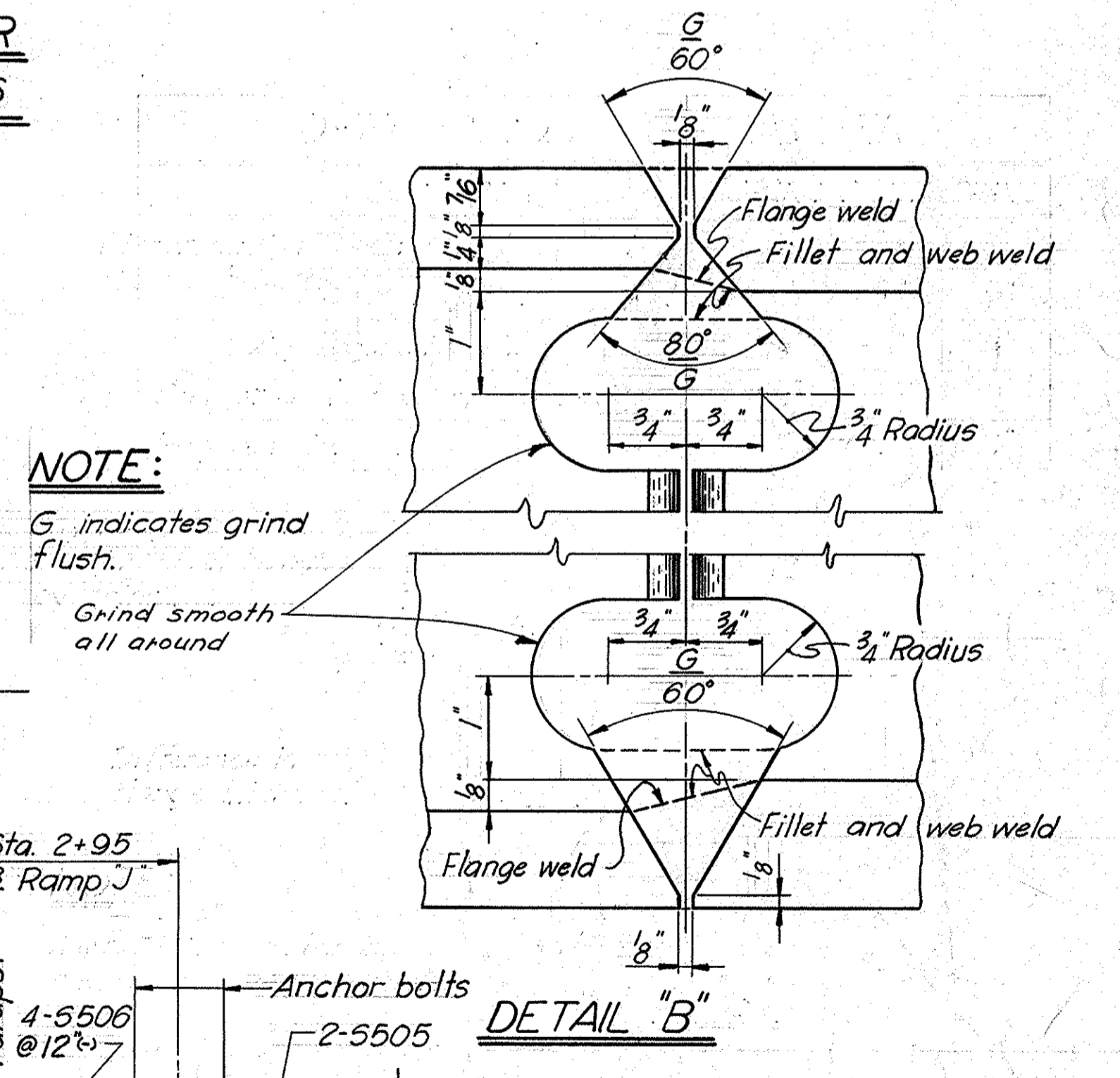
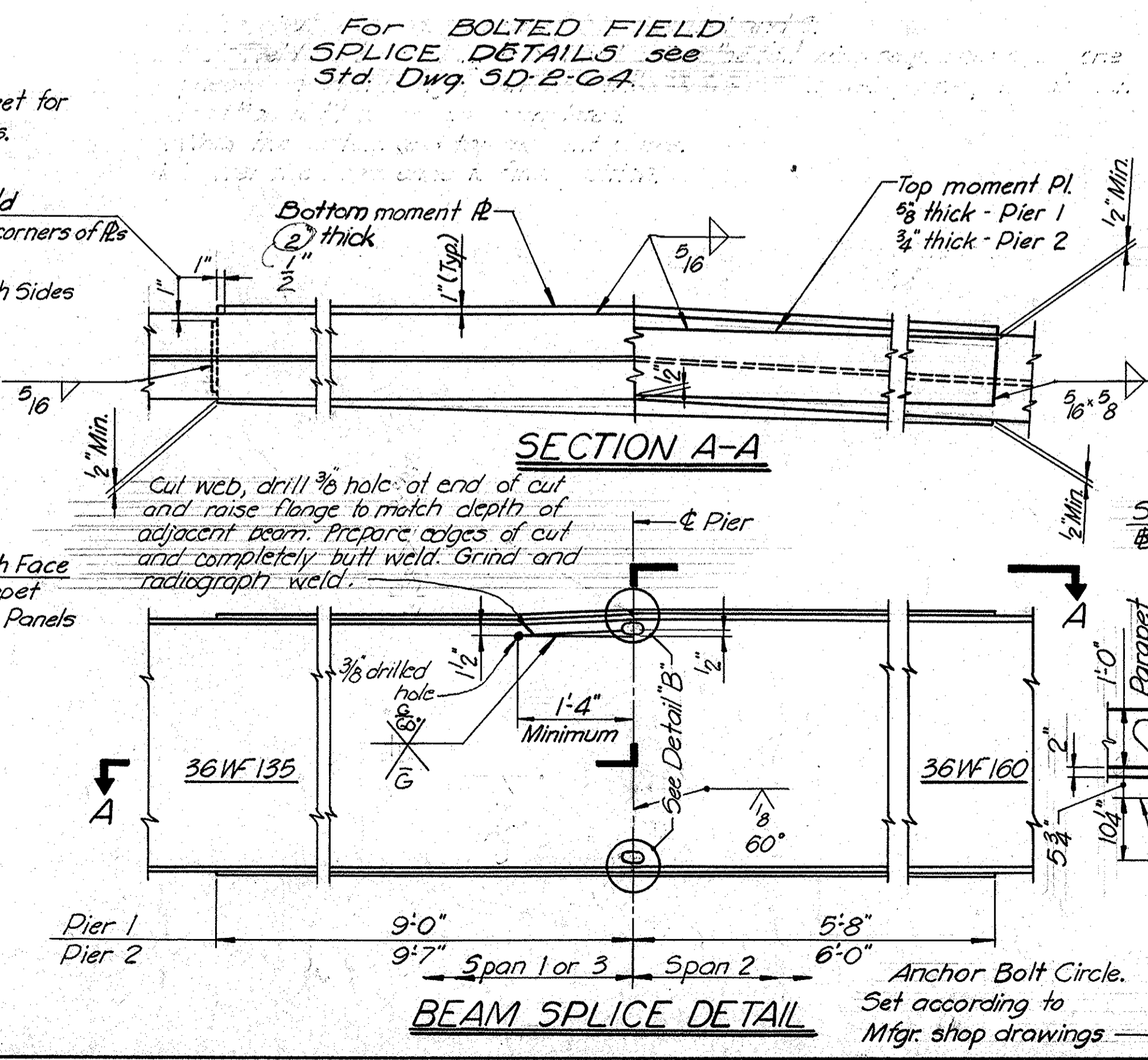
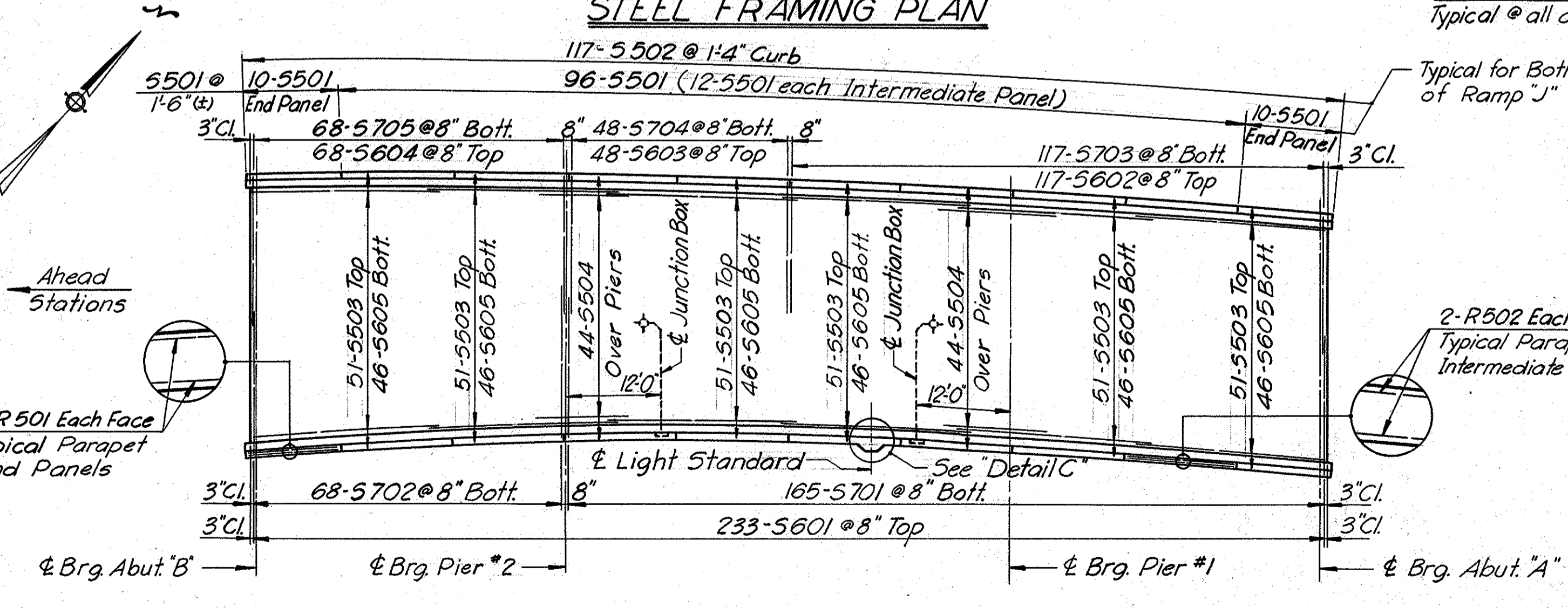
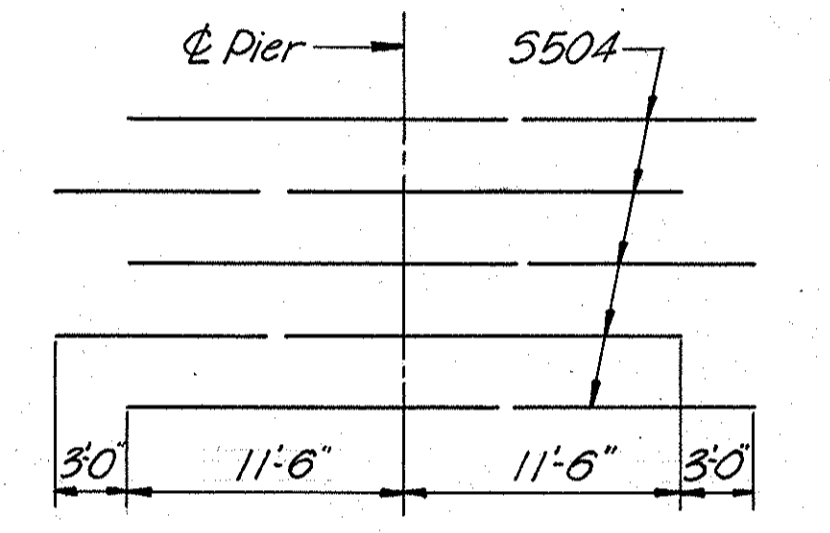


TABLE OF ANGLES

(A) 85°33'09"	(E) 88°05'54"	(J) 88°08'00"
(B) 88°39'27"	(F) 87°54'43"	(K) 88°58'54"
(C) 88°28'16"	(G) 86°26'22"	(L) 89°49'49"
(D) 88°17'05"	(H) 87°17'09"	



MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS
BRIDGE NO. MAH-18-RAMP J
OVER SALT SPRINGS ROAD

MAHONING COUNTY STA. 3+07.07

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
E.F.R.	E.R.J.	E.R.J.	R.A.L.	AK	8-26-63	8-19-65

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

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**MAH-18-15.50
MAHONING COUNTY**

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

BENCH MARK #211 Elev. 944.38 North Bolt of hydrant N.E. corner Salt Springs and Argo. 130' E. on Argo from Salt Springs.

BENCH MARK #212 Elev. 928.66 Top stem of hydrant N.E. corner Division and Salt Springs. 35' E. on Division from Salt Springs

ADT (1975) = 31,400

DIVISION STREET EXPRESSWAY OVER SALT SPRINGS ROAD

TYPE: Continuous steel beam with reinforced concrete deck and substructures.
SPANS: 46'-6", 66'-8", 46'-6"
ROADWAY: Varies: 72'-8" to 63'-8"
LOAD FREQUENCY: C.F. 2000 (57). Adequate for alternate A.A.S.H.O. loading.
SKEW: 16°03'48" R.F.
WEARING SURFACE: 1" Monolithic concrete.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Tangent and spiral.
SUPERELEVATION: Varies

RAMP J OVER SALT SPRINGS ROAD

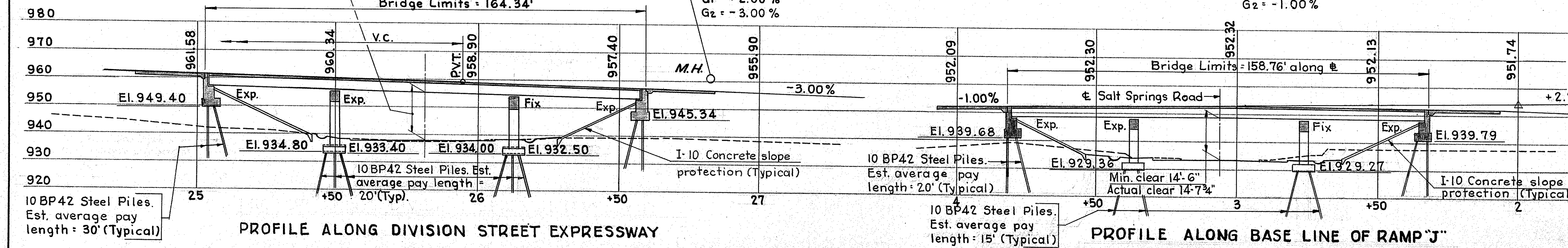
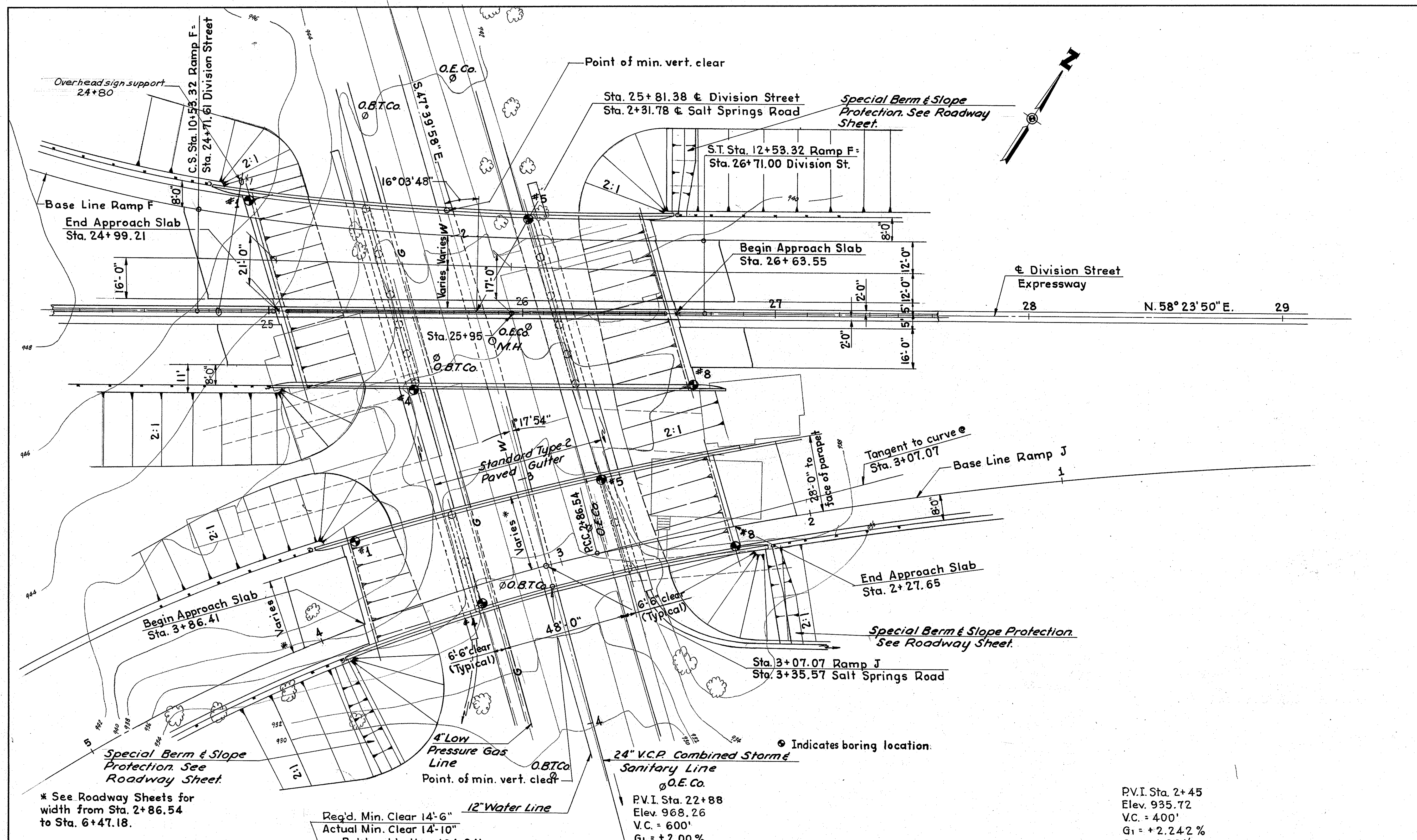
TYPE: Continuous steel beam with reinforced concrete deck and substructures.
SPANS: 45'-0", 64'-0", 45'-0"
ROADWAY: Varies: 33'-8" to 36'-10"
LOAD FREQUENCY: C.F. 2000 (57). Adequate for A.A.S.H.O. alternate loading.
SKEW: 1°17'54" R.F. to the local tangent.
WEARING SURFACE: 1" Monolithic concrete.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Curve
SUPERELEVATION: Varies.

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SITE PLAN

BRIDGE NO. MAH-18
DIVISION STREET EXPRESSWAY
OVER SALT SPRINGS ROAD

MAHONING COUNTY		STA. 25+81.38	
Designed	Drawn	Traced	Checked
FR	FR	R.T.R.	FR
			Reviewed
			Date
			10-14-63



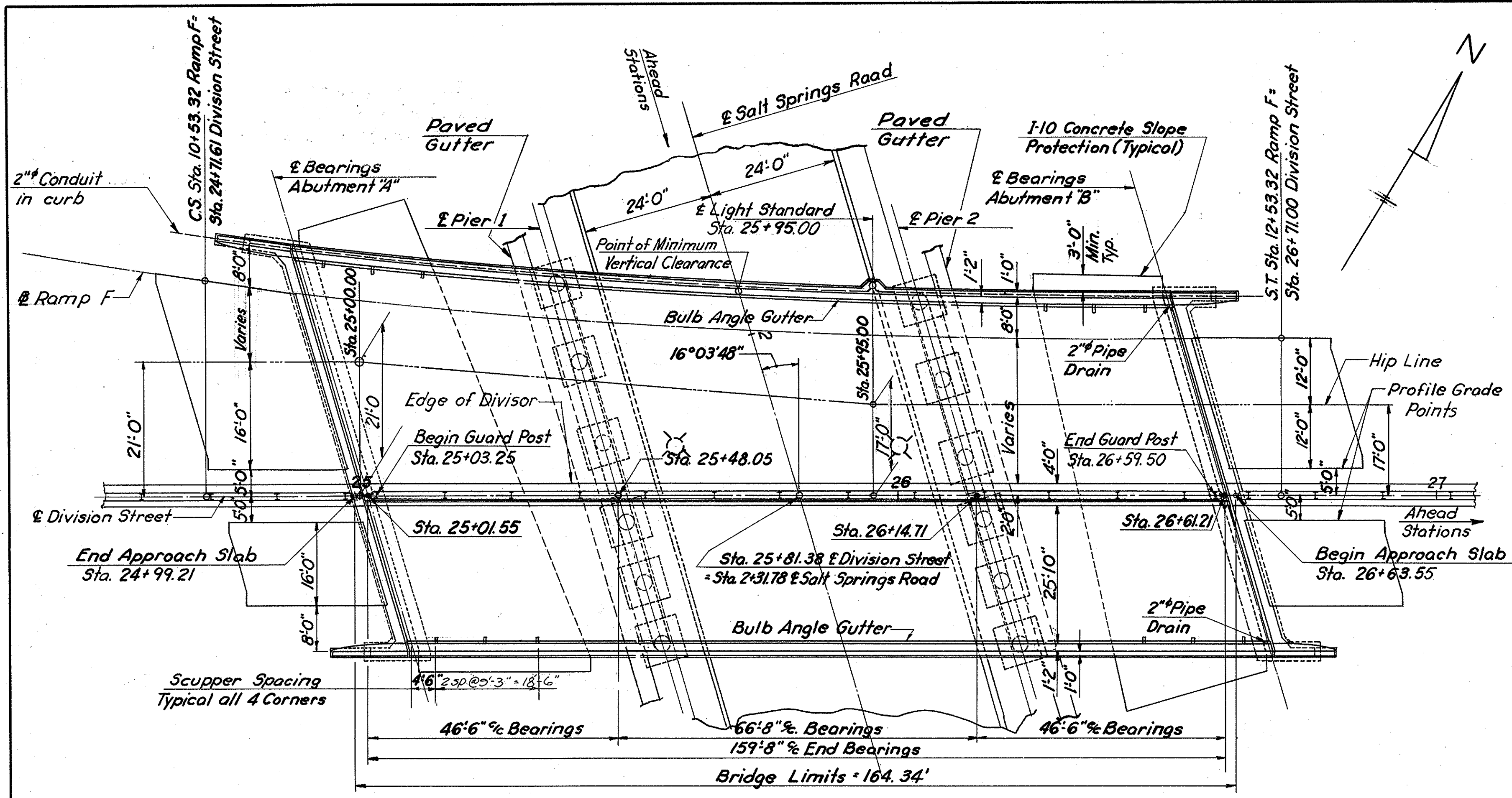
* See Roadway Sheets for width from Sta. 2+86.54 to Sta. 6+47.18.

Indicates boring location.
 P.V.I. Sta. 22+88
 Elev. 968.26
 V.C. = 600'
 G₁ = +2.00%
 G₂ = -3.00%

P.V.I. Sta. 2+45
 Elev. 935.72
 V.C. = 400'
 G₁ = +2.242%
 G₂ = -1.00%

PROFILE ALONG DIVISION STREET EXPRESSWAY

PROFILE ALONG BASE LINE OF RAMP "J"



PLAN

* ROADWAY ITEM FOR PAYMENT.

Indicates under bridge Luminaires.

LAYOUT OFFSETS & ELEVATIONS

STATION	GUTTER LINE		HIP		EDGE OF TOP OF PARAPET	
	DISTANCE	ELEV.	ELEV.	ELEV.	DIVISOR	PARAPET
24+71.61	47.44'	960.67	961.94	962.19	962.23	
+80	46.04	960.60	961.76	962.01	962.06	962.95
+90	44.52	960.49	961.55	961.80	961.85	962.84
25+00	43.16	960.36	961.33	961.58	961.62	962.72
+10	41.94	960.21	961.10	961.35	961.39	962.58
+20	40.88	960.04	960.87	961.11	961.15	962.41
+30	39.94	959.85	960.63	960.86	960.91	962.22
+40	39.12	959.65	960.38	960.60	960.65	962.02
+50	38.43	959.43	960.12	960.34	960.39	961.79
+60	37.84	959.20	959.86	960.07	960.11	961.55
+70	37.34	958.96	959.58	959.79	959.83	961.31
+80	36.94	958.71	959.30	959.50	959.54	961.07
+90	36.61	958.45	959.01	959.20	959.25	960.82
26+00	36.36	958.20	958.72	958.90	958.94	960.57
+10	36.16	957.96	958.45	958.60	958.64	960.31
+20	36.03	957.72	958.17	958.30	958.33	960.07
+30	35.93	957.48	957.90	958.00	958.03	959.82
+40	35.88	957.23	957.62	957.70	957.72	959.58
+50	35.85	956.99	957.35	957.40	957.41	959.34
+60	35.84	956.73	957.06	957.10	957.11	959.08
+70	35.83	956.50	956.80	956.80	956.80	

FOR LEFT STRUCTURE ONLY

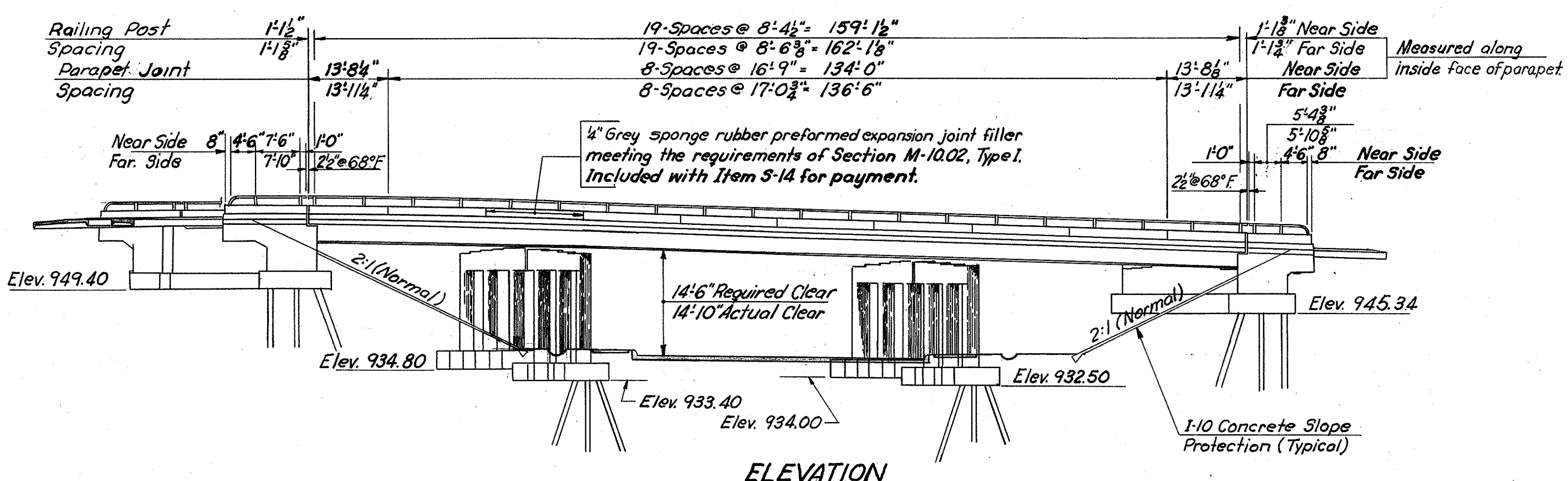
MAH-18-15.50
MAHONING COUNTY

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

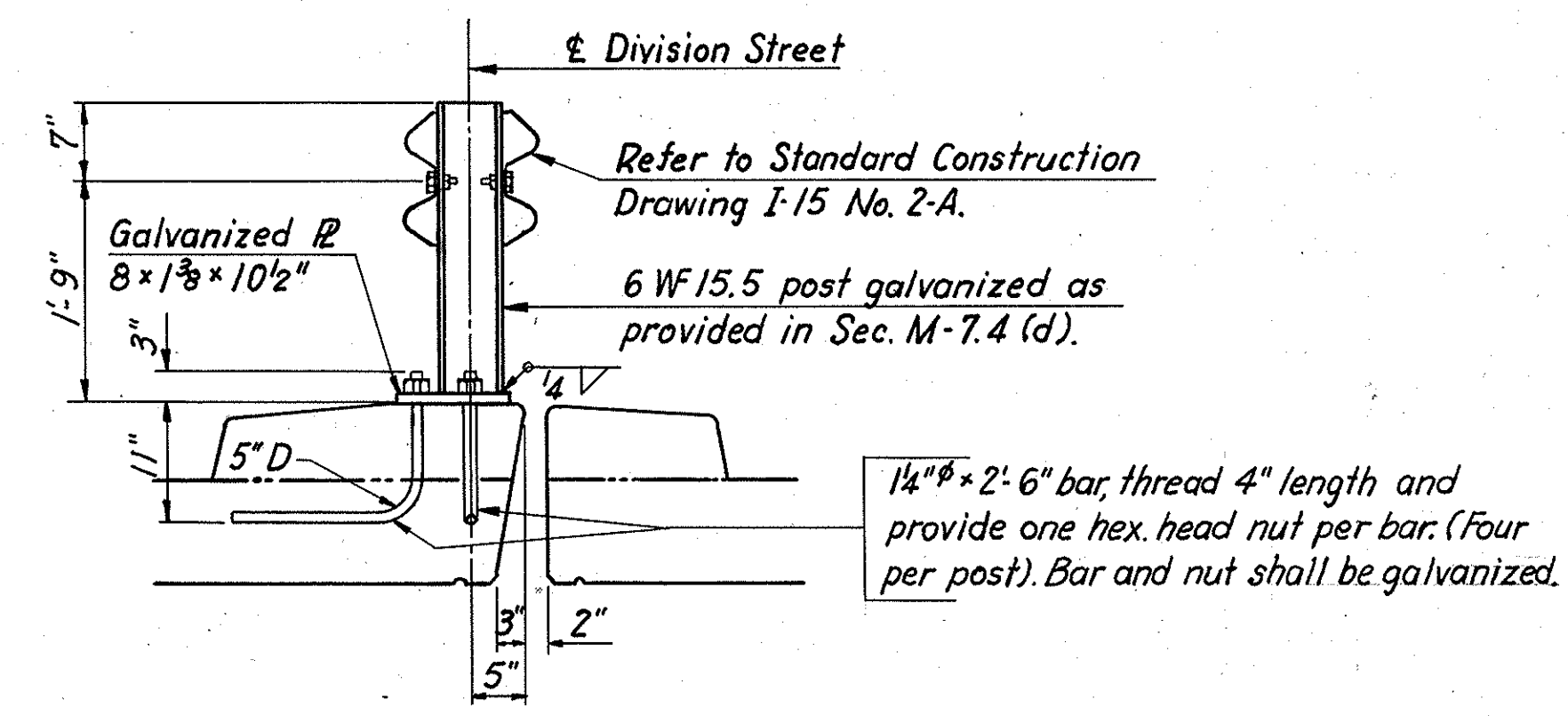
247

GENERAL NOTES

- DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.
- REFERENCE shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59; AR-1-57 revised 4-2-62; FSB-1-62, revised 1-15-63; Supplemental Specifications S10I dated 12-2-59, and AS-1-54 revised 7-5-62.
- DESIGN LOADING - CF 2,000 (57).
- CLASS C CONCRETE - Basic unit stress 1,333 p.s.i.
- CLASS E CONCRETE - Basic unit stress 1,133 p.s.i.
- STRUCTURAL STEEL - ASTM A36-basic unit stress 20,000 p.s.i. (ASTM A7 and A373 steel not permitted).
- REINFORCING STEEL - ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 p.s.i. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 p.s.i.
- EXCAVATION QUANTITY includes the removal of fill material required for construction of the abutments.
- PILES shall be driven to a minimum bearing capacity of 33 tons per pile for the abutments and 33 tons per pile for the piers.
- WELDING of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.
- MACHINE FINISH: The right portion of the bridge deck shall be finished by the use of a finishing machine. At the Contractor's option, the left portion of the bridge deck may be finished by the use of a finishing machine.
- REFERENCE shall be made to Supplemental Specification 5-307 revised 10-1-64 and Standard Drawing 5D-2-64 dated 11-25-64



ELEVATION



MEDIAN BARRIER GUARD RAIL DETAILS

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

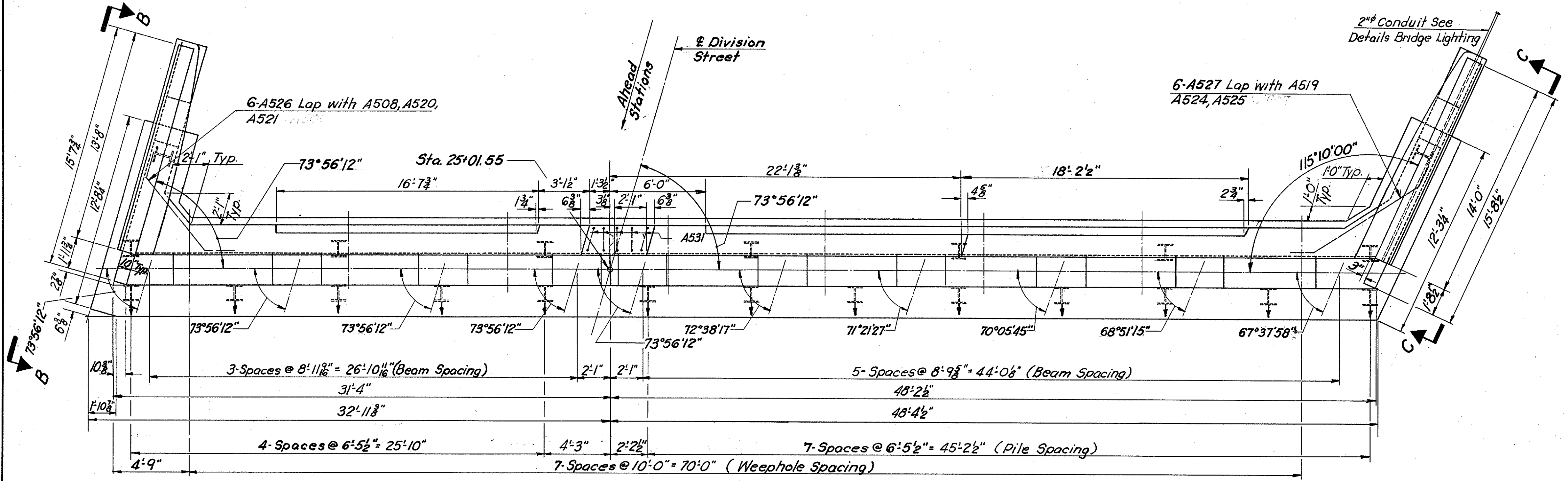
GENERAL PLAN & ELEVATION

BRIDGE NO. MAH-18
DIVISION STREET EXPRESSWAY
OVER SALT SPRINGS ROAD

MAHONING COUNTY STA 25+81.38

Designed	Drawn	Traced	Checked	Reviewed Date	Revised
EFR	R ₁	R ₁	D.M.	F.K. 10-14-63	

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	



NOTES:
Porous backfill 2feet thick full length of abutment shall extend up to the underside of the approach slab and outward to the wings. Excavation therefor, in excess of that required for construction of the abutment, shall be considered as paid for in the bid price per cu. yd. paid for porous backfill.

All parapet concrete shall be Class "C".

Concrete and reinforcing steel above parapet construction joint shall be included with railing S-14 for payment.

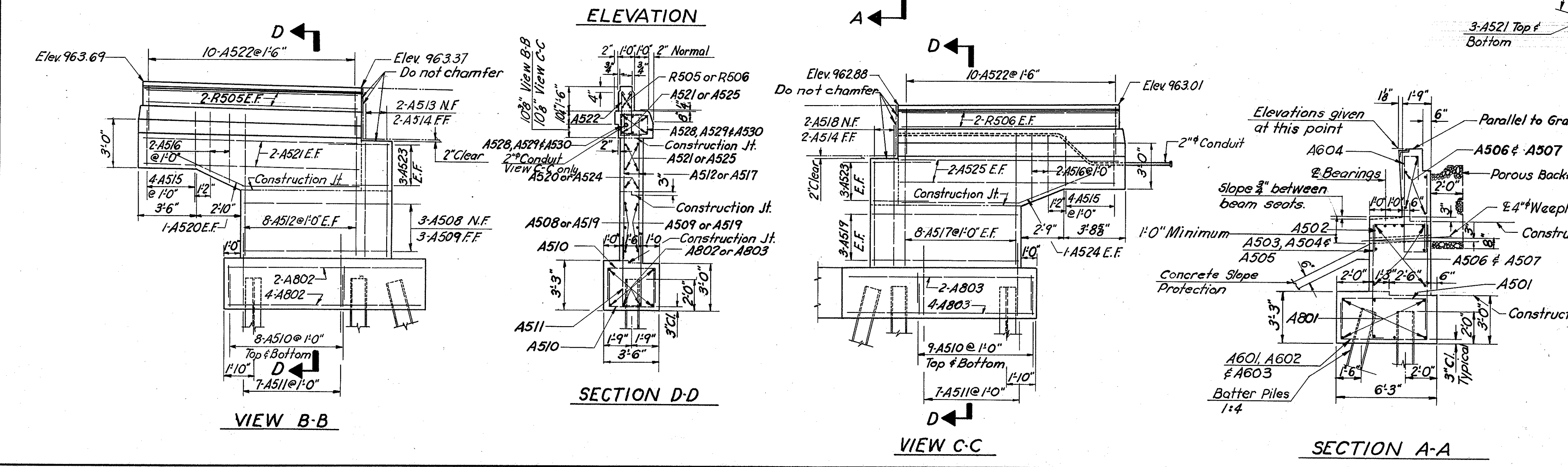
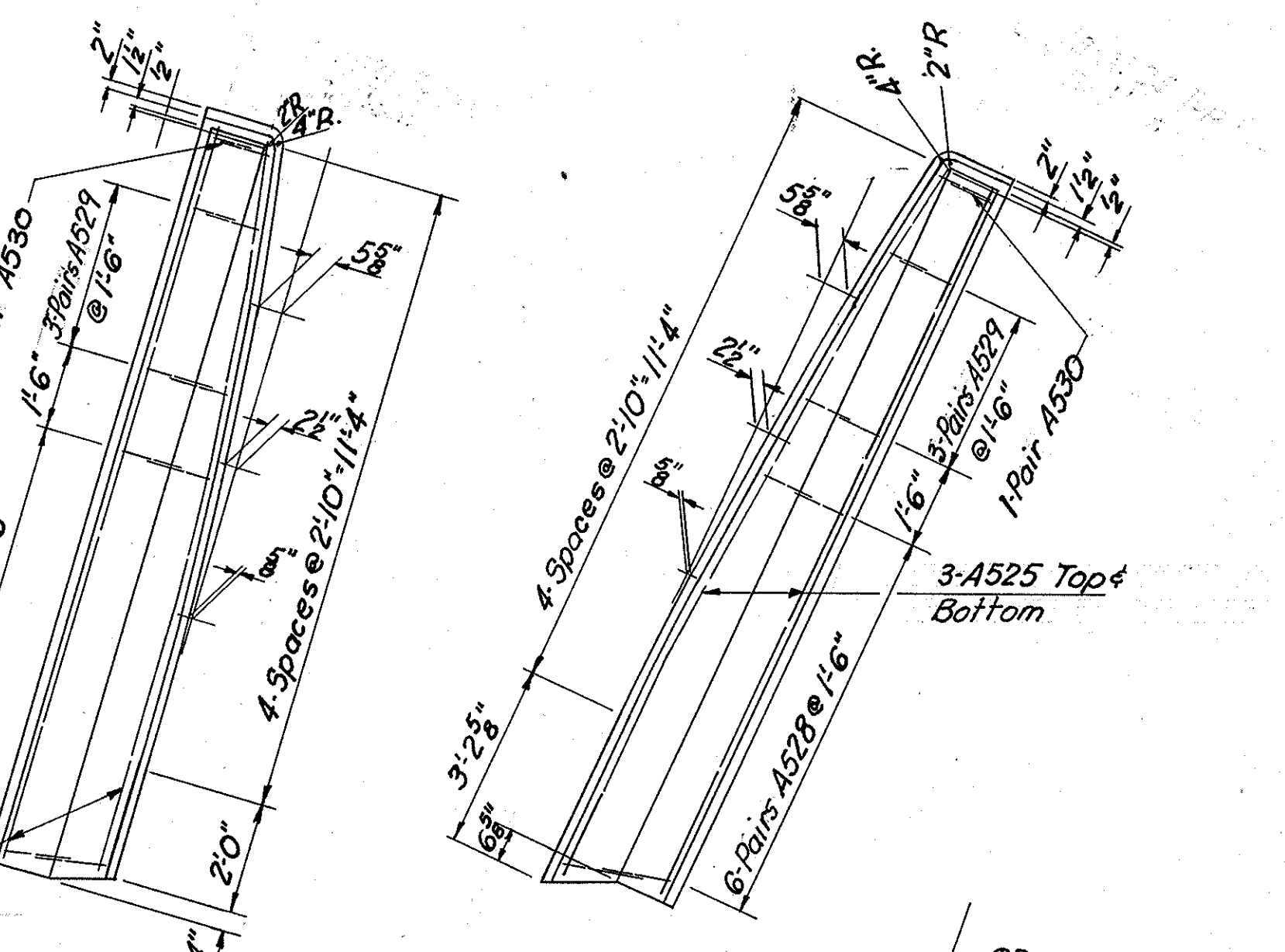
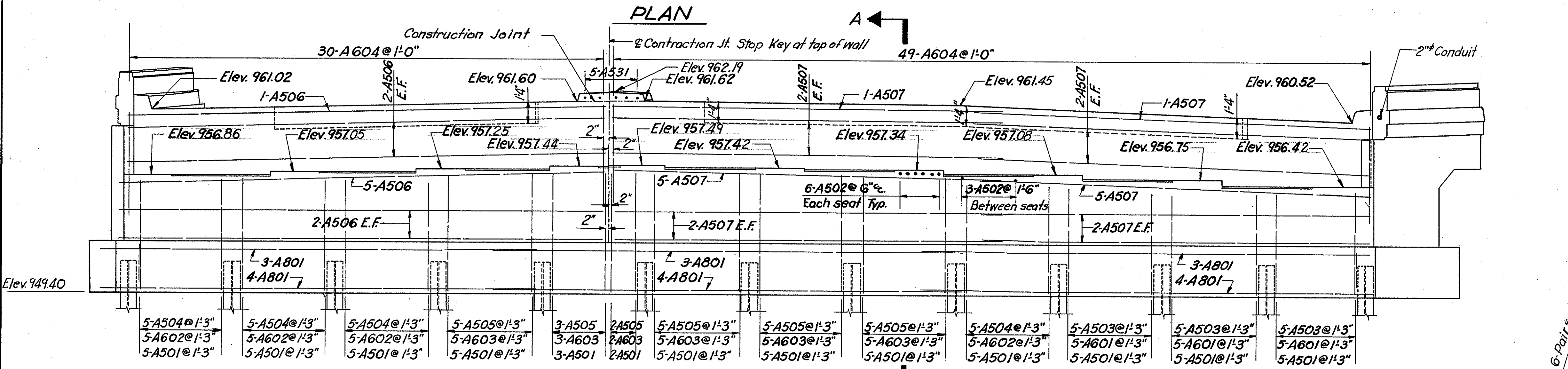
All abutment parapet railings shall be continuous through railing posts.

Refer to Standard Drawing AR-1-57 (Revised 4-2-62) for details of Type "A" railing and concrete parapet.

PROCEDURE: The embankment shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade for a distance of 200 feet back of the abutments after which the excavation shall be made for the abutments and piles driven.

BRIDGE SEAT REINFORCING: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor holes.

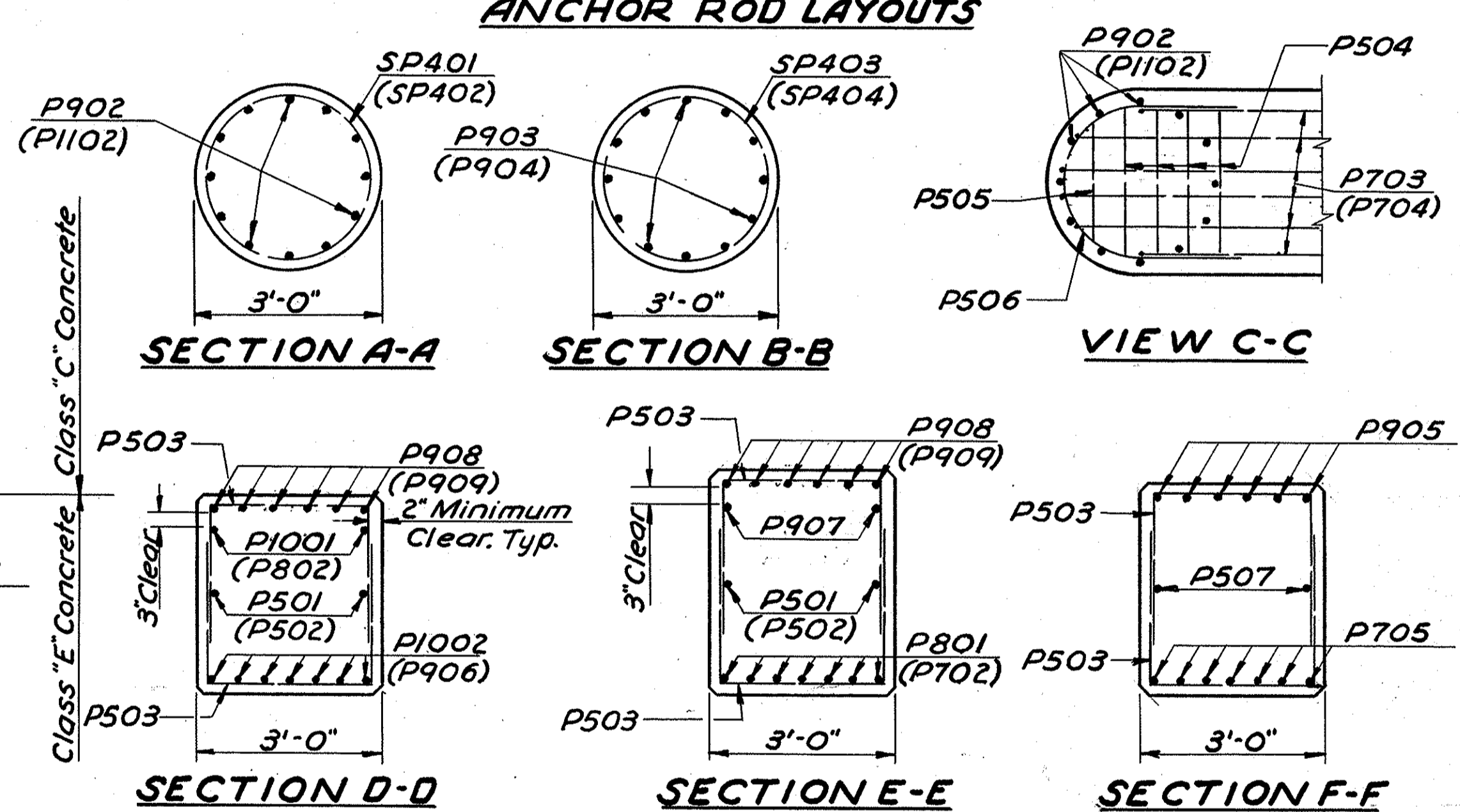
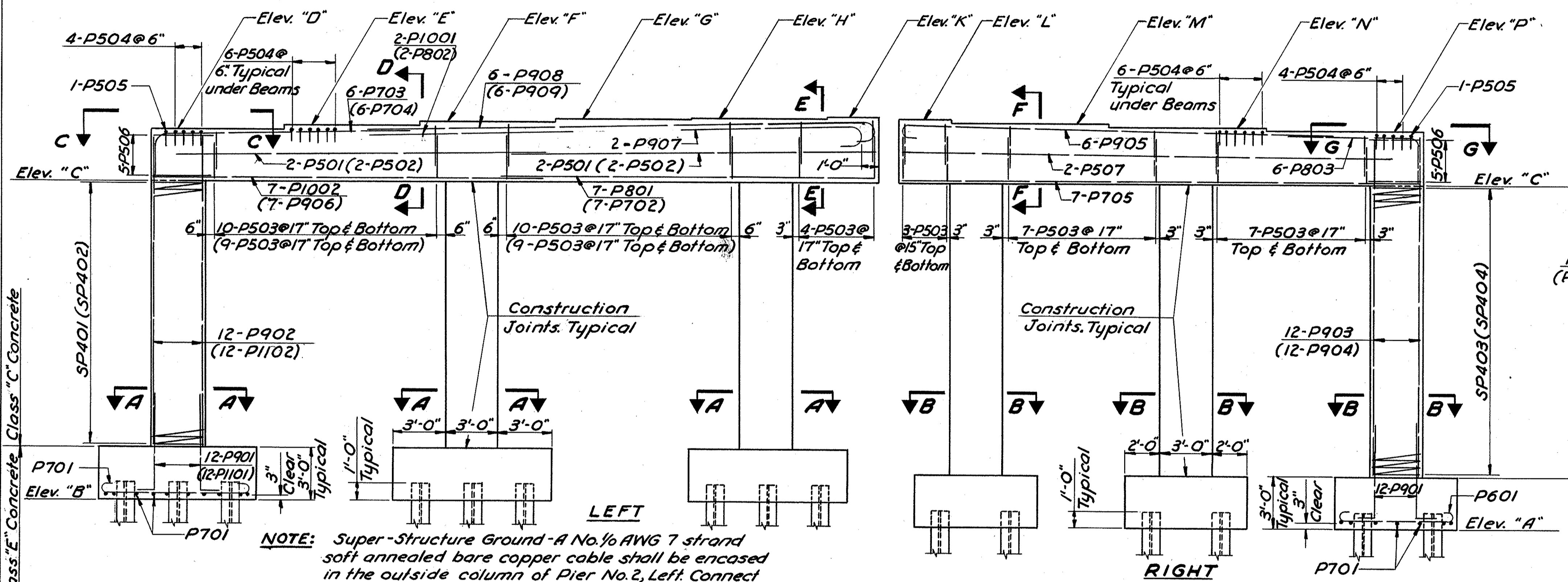
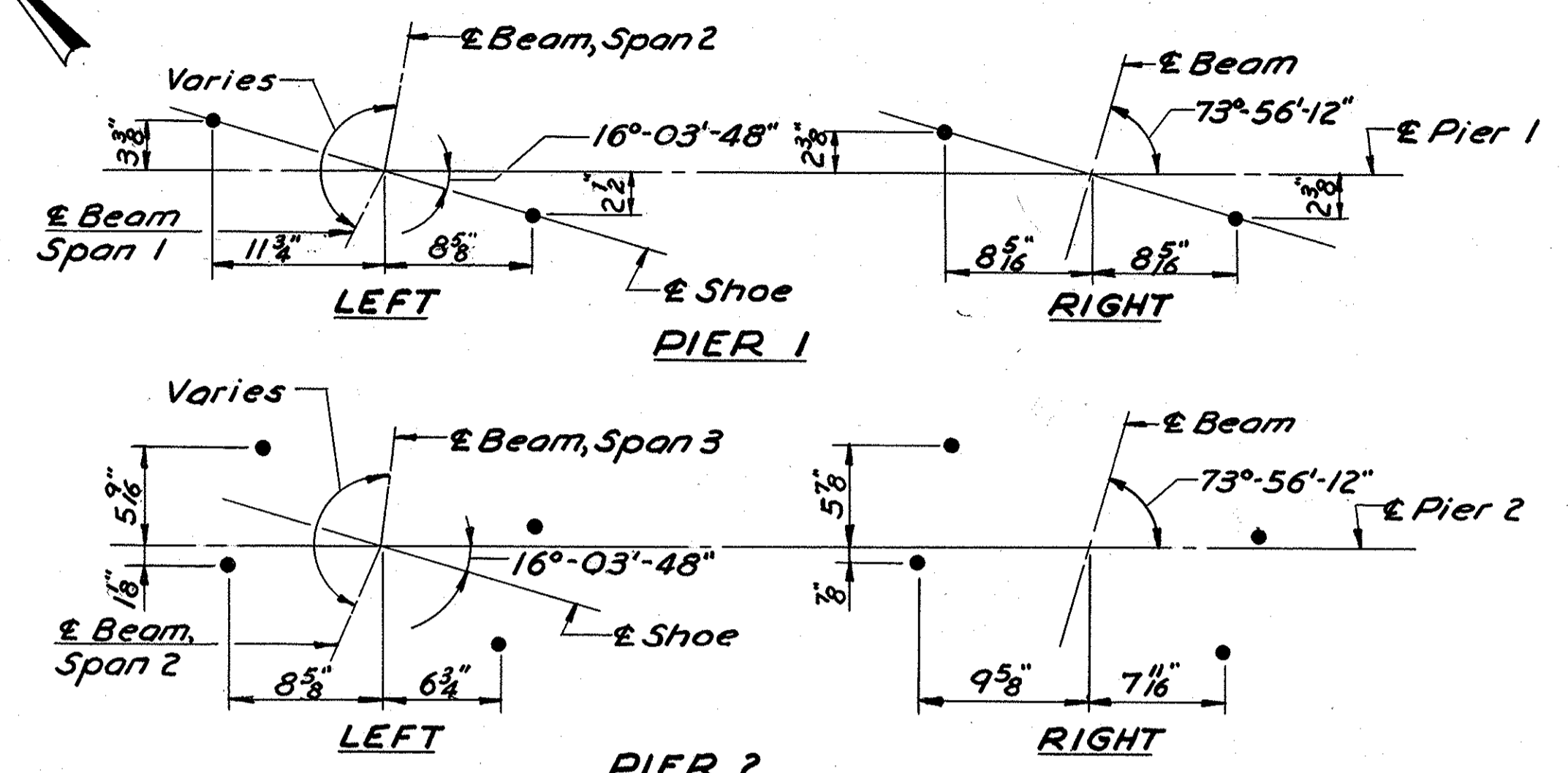
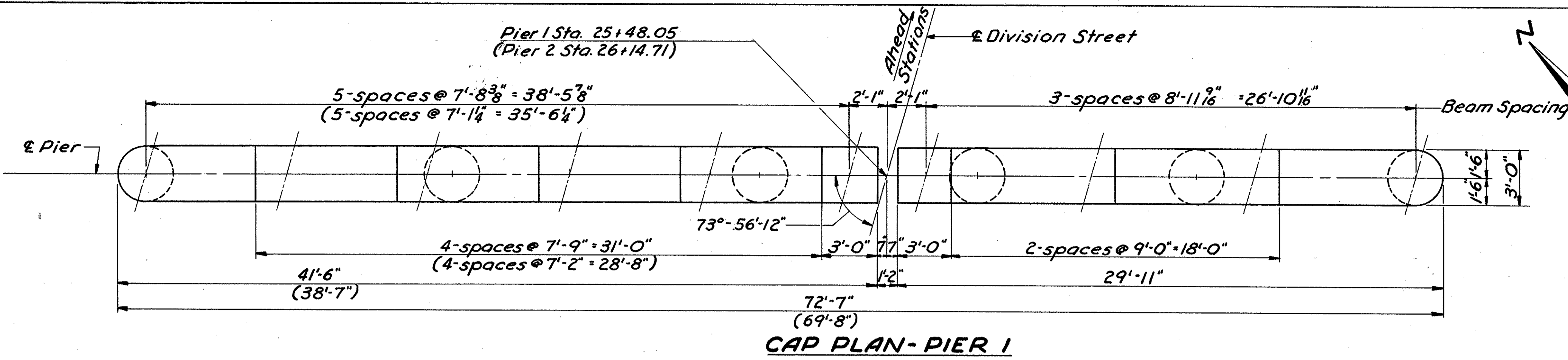
All abutment concrete shall be Class "E".



LEGEND
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA				
ABUTMENT A				
BRIDGE NO. MAH-18 DIVISION STREET EXPRESSWAY OVER SALT SPRINGS ROAD				
MAHONING COUNTY		STA. 25+81.38		
Designed	Drawn	Traced	Checked	Reviewed Date
DUN.	DUN.	R	EFR	F.K. 10-14-63

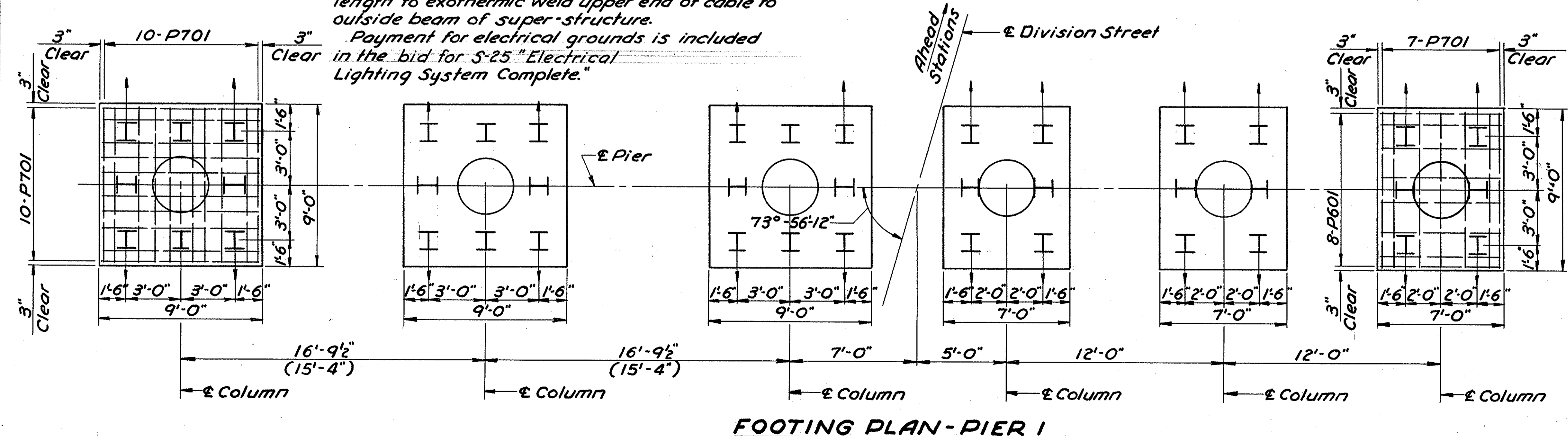
MAH-18-15.50
MAHONING COUNTY



NOTE: Super-Structure Ground-A No. 10 AWG 7 strand soft annealed bare copper cable shall be encased in the outside column of Pier No. 2, Left. Connect (by Exothermic weld process) lower end of cable to a steel pile. Extend cable (in one continuous length) through top of pier with lead of sufficient length to exothermic weld upper end of cable to outside beam of super-structure. Payment for electrical grounds is included in the bid for S-25 "Electrical Lighting System Complete."

TABLE OF ELEVATIONS

Elevation	Pier 1	Pier 2
A	933.40	932.50
B	934.80	934.00
C	952.58	950.39
D	955.58	953.74
E	955.80	953.87
F	956.02	953.99
G	956.18	954.10
H	956.24	954.13
K	956.30	954.15
L	956.24	954.02
M	956.04	953.81
N	955.84	953.60
P	955.64	953.39



NOTES:
 • Pier 1 details are shown. Pier 2 details similar except where noted thus ().
 • Piles shown thus () are to be battered 1:4 in direction indicated by arrow.
 • Special care shall be taken in placing reinforcing steel in the pier cap so that it will not interfere with the drilling of anchor rod holes.

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ROCHESTER, PENNSYLVANIA

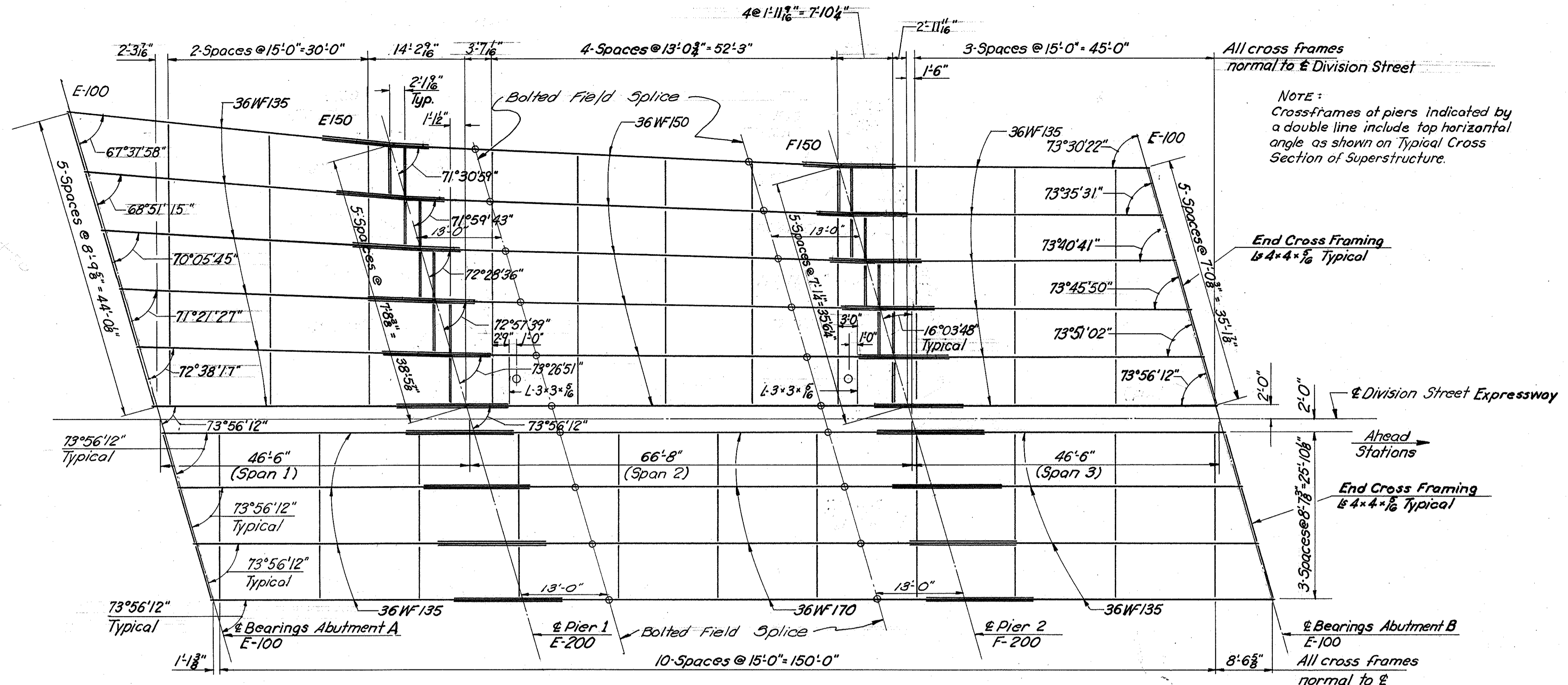
PIERS

BRIDGE NO. MAH-18
DIVISION STREET EXPRESSWAY
OVER SALT SPRINGS ROAD

MAHONING COUNTY STA. 25+81.38

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
		P.D.	DUN.		10-14-63	

MAH-18-1550
MAHONING COUNTY



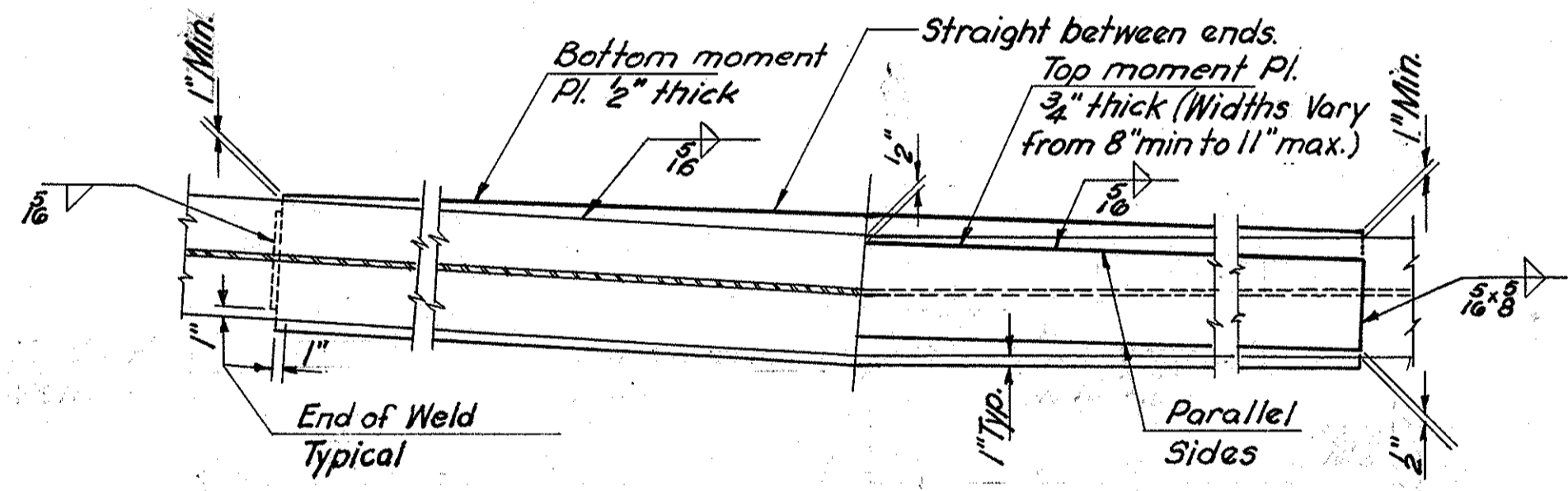
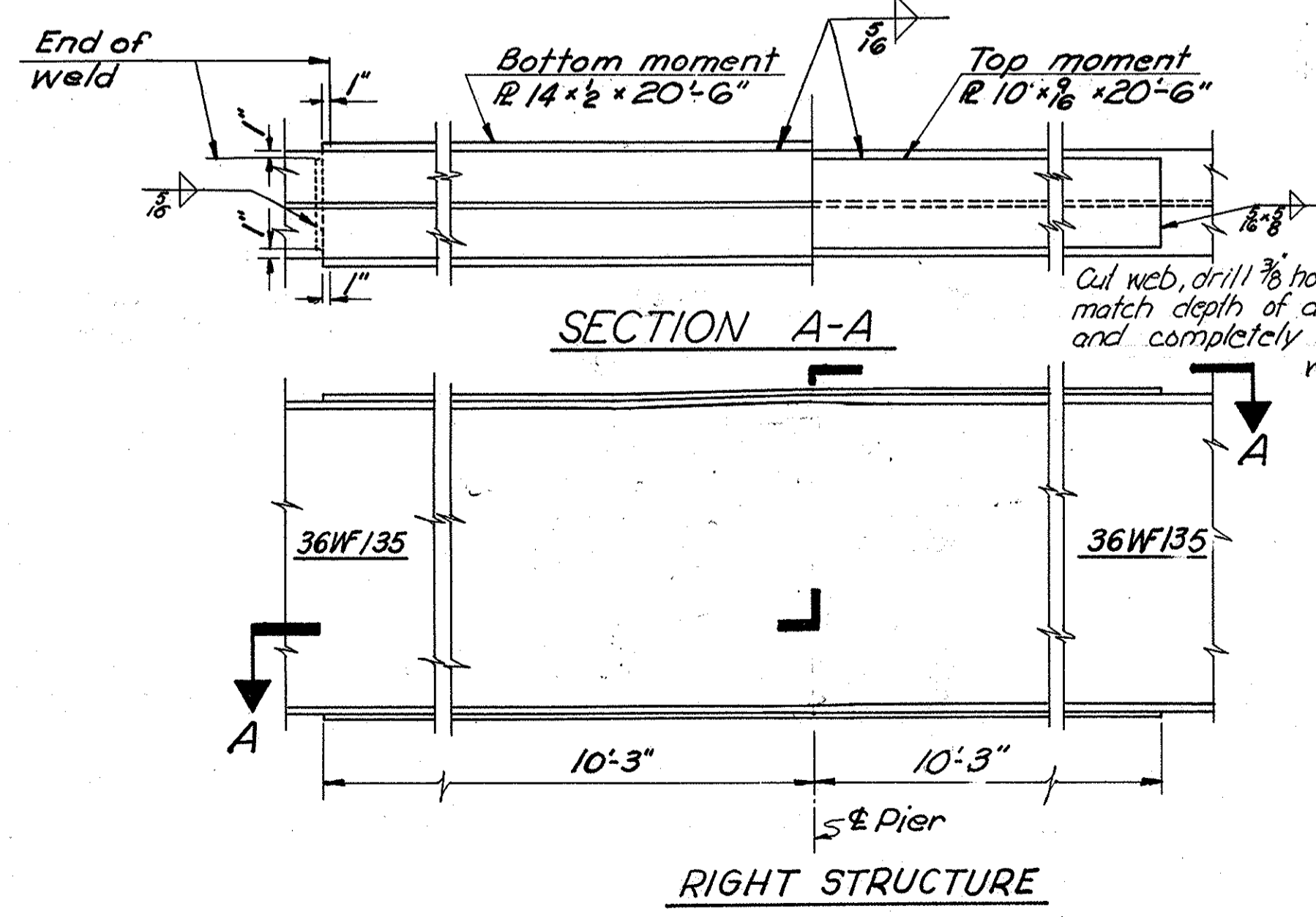
All cross frames normal to Division Street

NOTE: Crossframes of piers indicated by a double line include top horizontal angle as shown on Typical Cross Section of Superstructure.

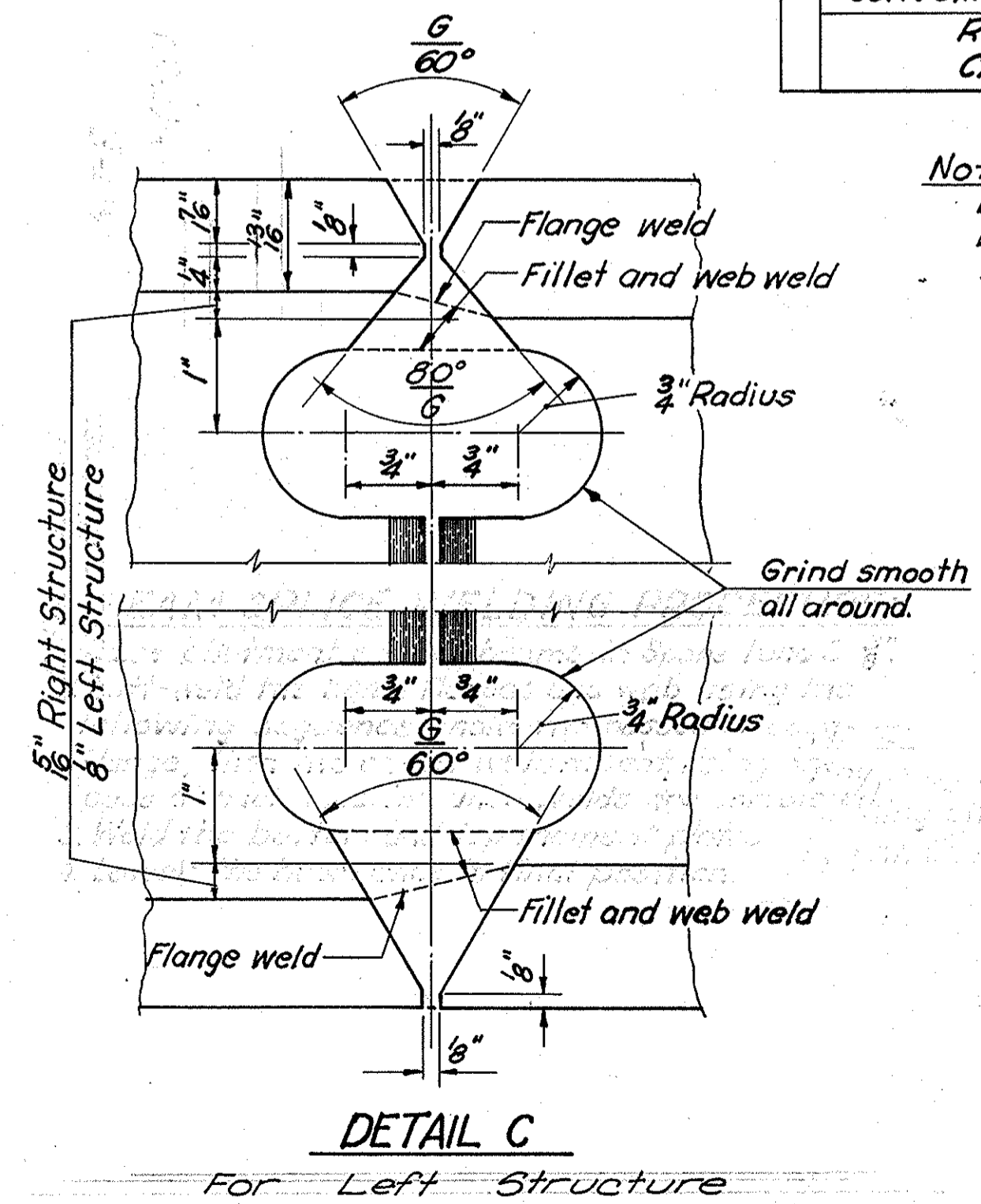
DEFLECTION AND CAMBER						
LOCATION	RIGHT STRUCTURE			LEFT STRUCTURE		
	SPAN 1	SPAN 2	SPAN 3	SPAN 1	SPAN 2	SPAN 3
INTERIOR	Deflection due to weight of steel	0	1/16"	0	0	1/16"
	Deflection due to weight of remaining dead load	1/8"	3/8"	1/8"	1/8"	1/8"
	Convexity required for vertical curve	4"	5/16"	0	2"	3/8"
	Sum of deflection and convexity	3/8"	3/4"	1/8"	5/8"	3/4"
	REQUIRED CAMBER	0	3/4"	0	0	3/8"
EXTERIOR	Deflection due to weight of steel	0	1/16"	0	0	1/16"
	Deflection due to weight of remaining dead load	1/8"	3/8"	1/8"	1/8"	1/8"
	Convexity required for vertical curve	4"	5/16"	0	7/16"	3/8"
	Sum of deflection and convexity	3/8"	3/4"	1/8"	7/16"	3/8"
	REQUIRED CAMBER	0	3/4"	0	0"	3/8"
MEDIAN	Deflection due to weight of steel	0	1/16"	0	0	1/16"
	Deflection due to weight of remaining dead load	1/8"	5/16"	1/8"	1/8"	1/16"
	Convexity required for vertical curve	4"	3/8"	0	4"	9/16"
	Sum of deflection and convexity	3/8"	3/8"	1/8"	3/8"	16"
	REQUIRED CAMBER	0	3/4"	0	0"	3/8"

STEEL FRAMING PLAN

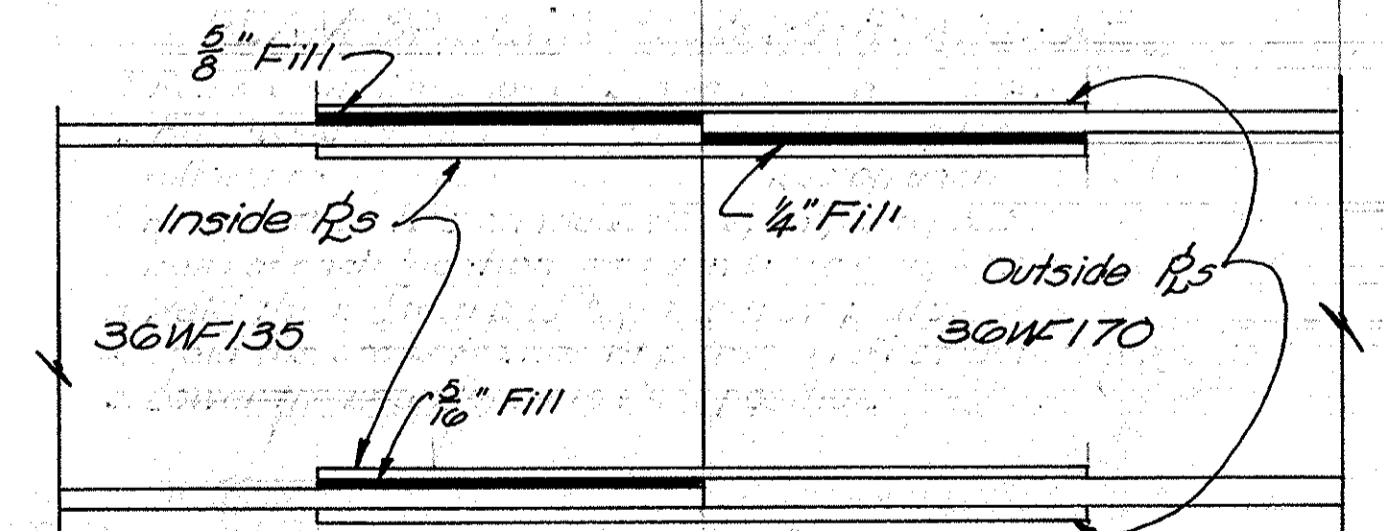
NOTES:
All shoes are set with their transverse axes normal to Division Street.
The width of the E-150 masonry plate for the left bridge is 2'-14". (See Pier Sheet for Anchor Rod Layout.)



BEAM SPLICE DETAILS



NOTES:
Where no camber is specified, beams shall be fabricated with any natural camber or bowed side up.
See Superstructure Details sheet for notes.



See Std. Dwg. 3D-2-64 for additional 36WF130 splice details.
36WF135 TO 36WF170 BEAM SPLICE DETAILS
See Std. Dwg. 3D-2-64 For all 36WF150 splice details

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SUPERSTRUCTURE DETAILS

BRIDGE NO. MAH-18
DIVISION STREET EXPRESSWAY
OVER SALT SPRINGS ROAD

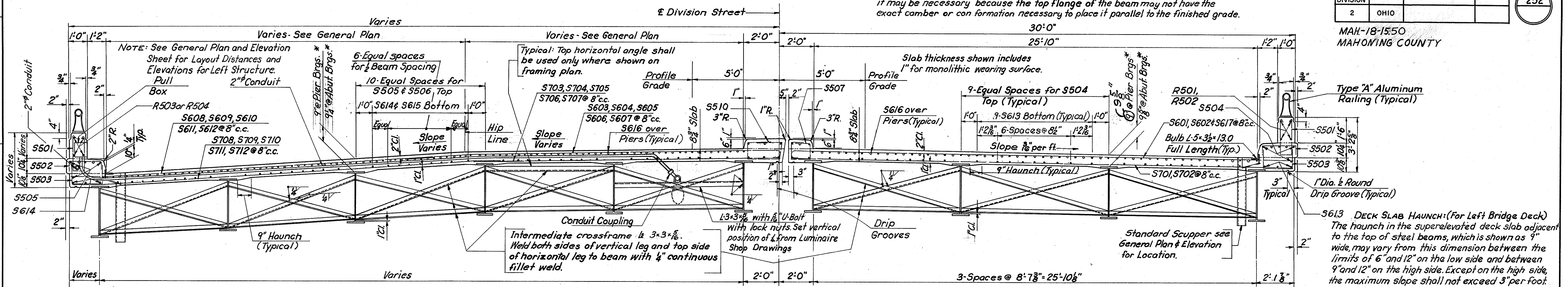
MAHONING COUNTY STA. 25 + 81.38

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
E.F.R.	R ₃	R ₃	D.W.	F.K.	10-14-63	

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

MAH-18-1550
MAHONING COUNTY

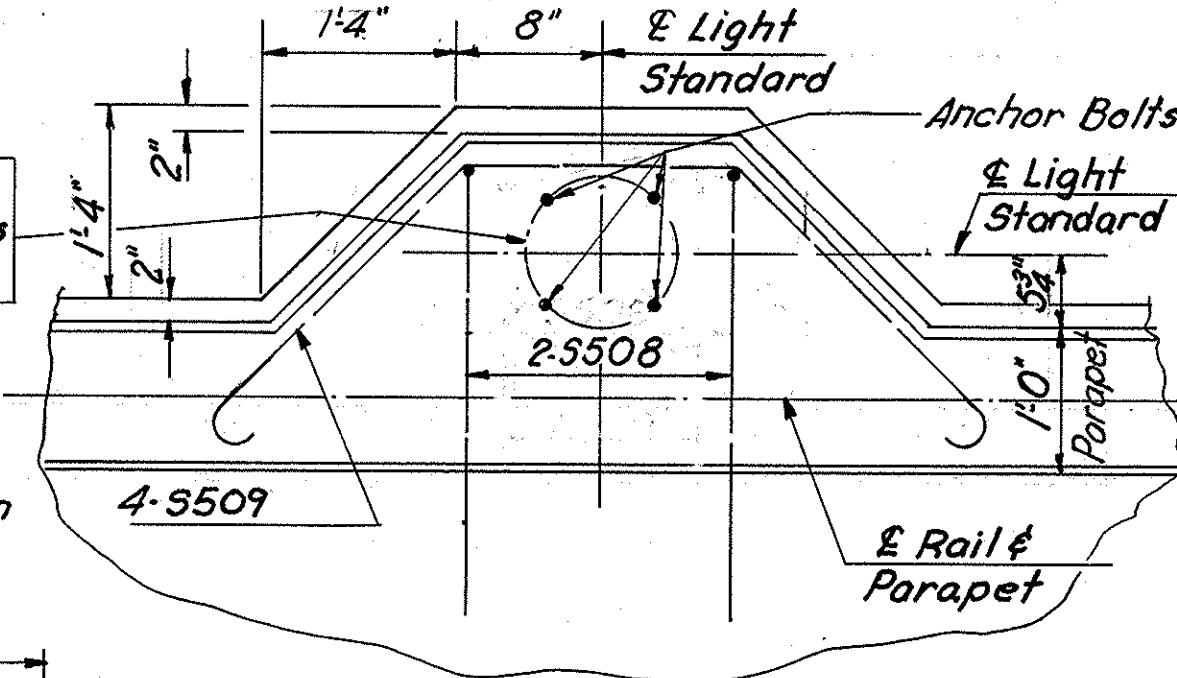
* This is the nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or con formation necessary to place it parallel to the finished grade.



TYPICAL CROSS SECTION

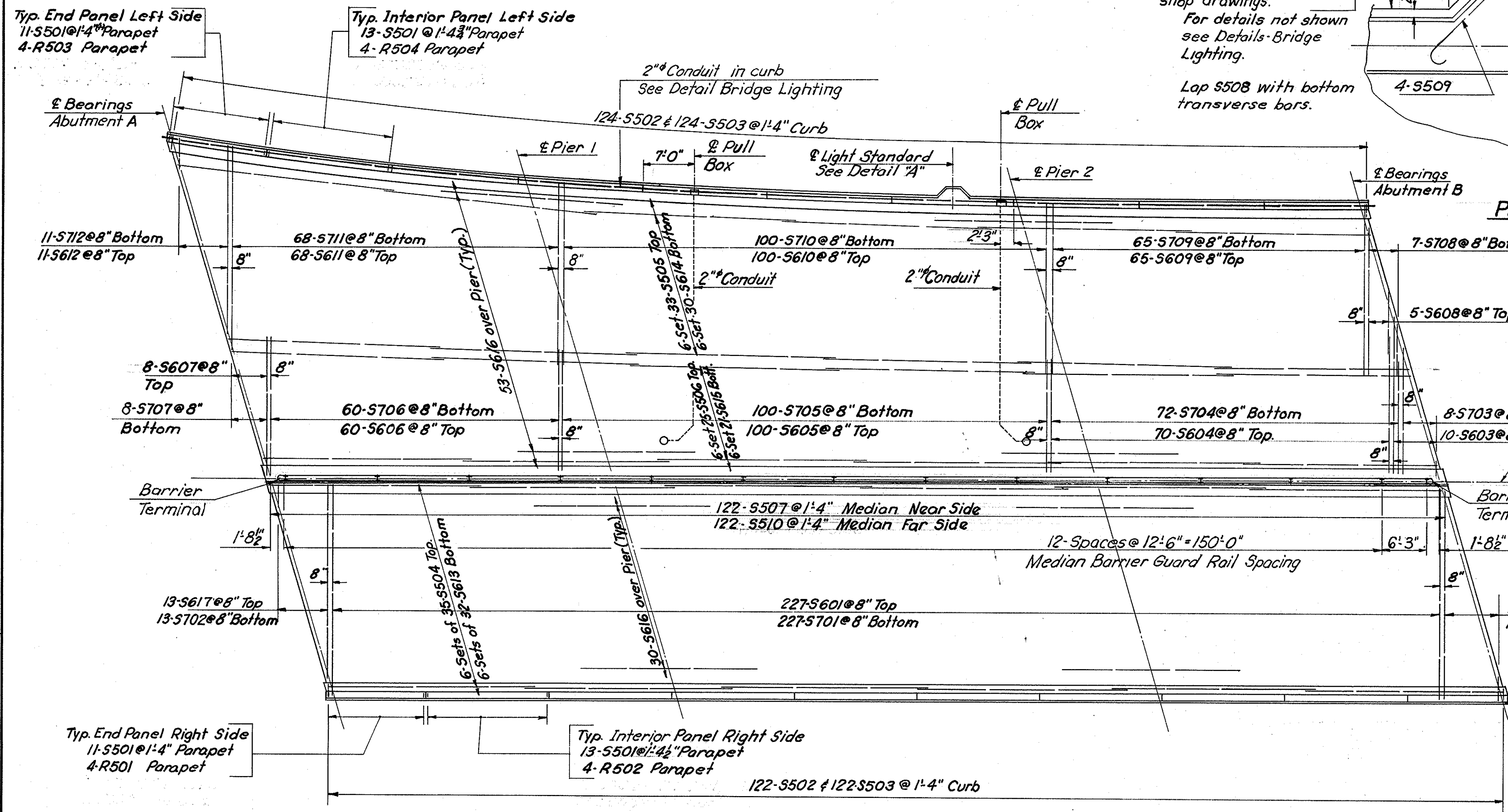
Anchor bolt circle. Set according to manufacturer's shop drawings. For details not shown see Details - Bridge Lighting.

Lop S508 with bottom transverse bars.



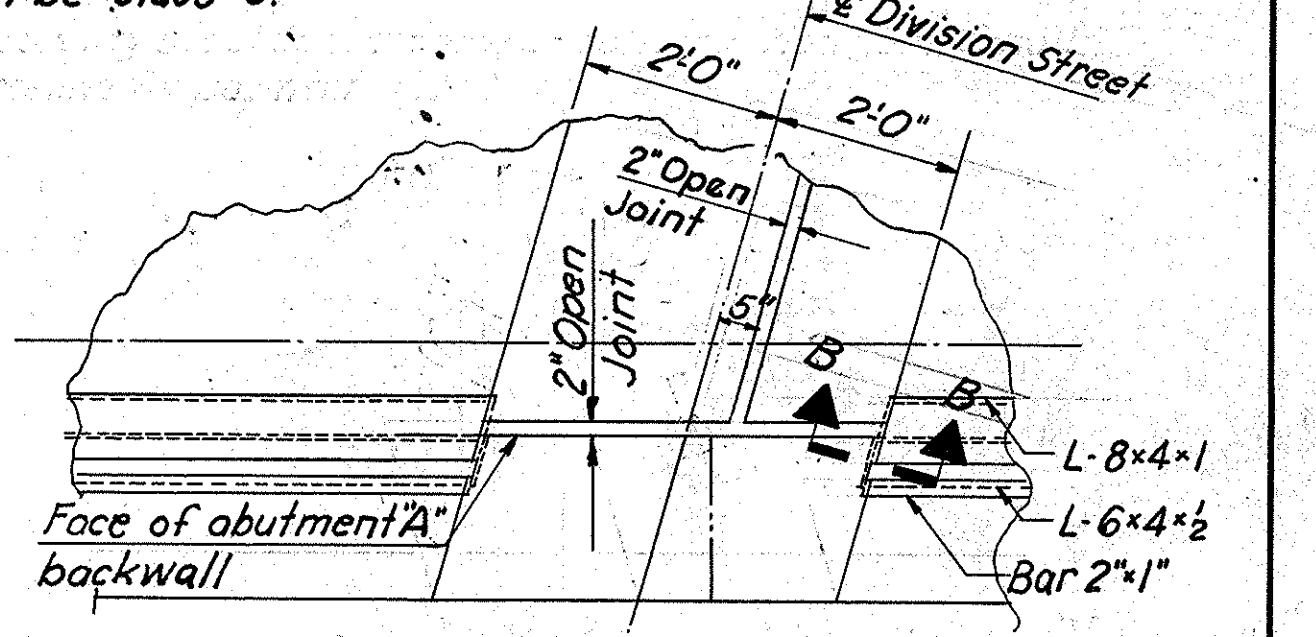
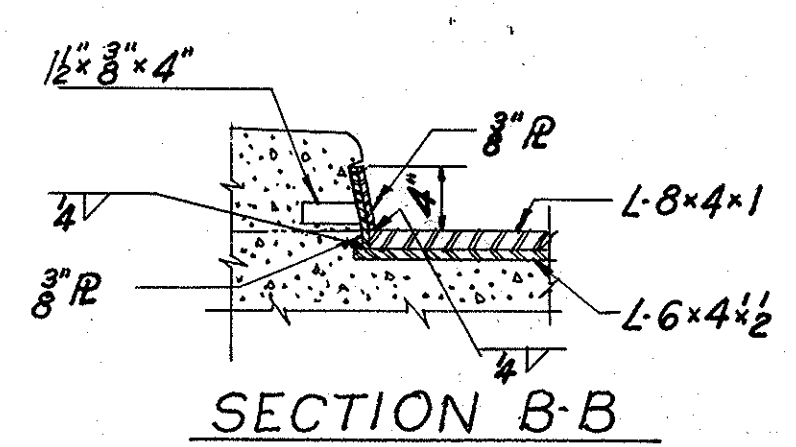
NOTES CONTINUED

- Refer to Standard Drawing AR-1-57 for details of Type 'A' Aluminum Railing and concrete parapet, to Standard Drawing CSB-2-56, sheets 2&3 of 6 for details of expansion dams, end crossframes, 2" pipe drains, curb plates and scuppers; and for bearings refer to Standard Drawing F.S.B.-1-62 and to abutment and pier sheets for anchor rod layout.
- See General Plan and Elevation for location of scuppers, 2" pipe drains and railing posts.
- CONCRETE DECK PLACING: In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress up grade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
- DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12". Except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width. (For Right Bridge Deck.)
- Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
- All concrete shall be Class "C".



DECK REINFORCING PLAN

DIAGRAM SHOWING STAGGER OF S616 BARS



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ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS

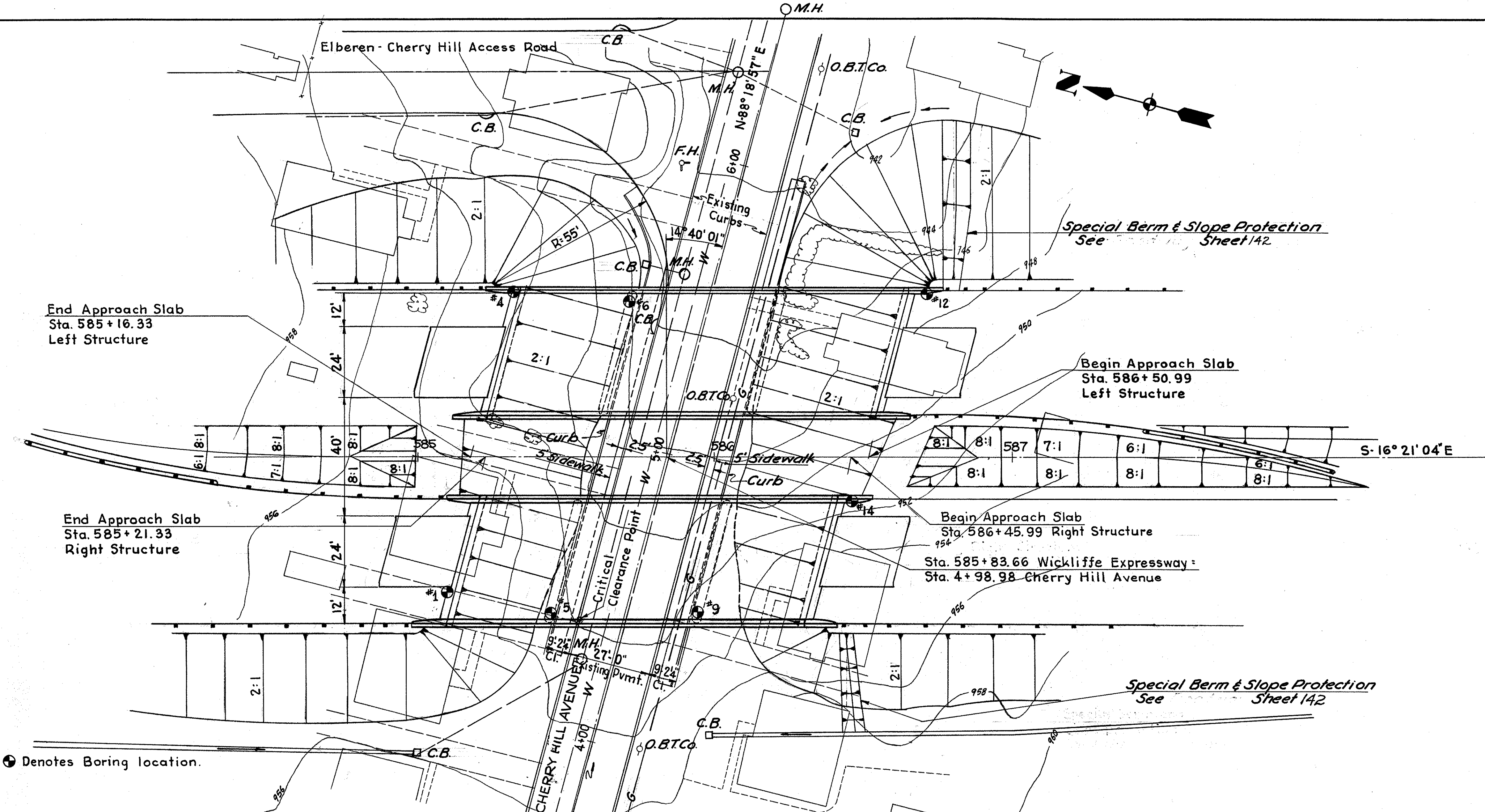
BRIDGE NO. MAH-18
DIVISION STREET EXPRESSWAY
OVER SALT SPRINGS ROAD

MAHONING COUNTY		STA. 25+81.38	
Designed	Drawn	Traced	Checked
EFR	Ry	Ry	DM
Reviewed Date	10-14-63	Revised	4-7-66

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

254

**MAH-18-15.50
MAHONING COUNTY**

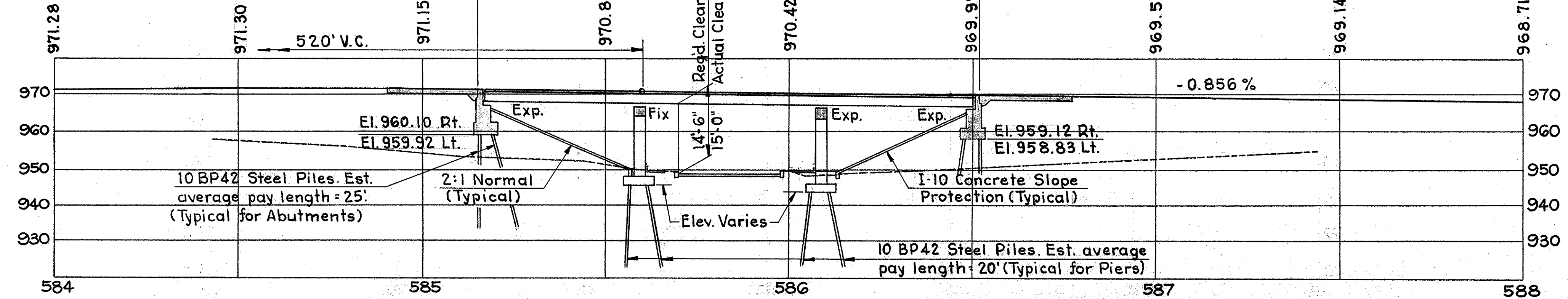


B.M. 138 Top stem of hydrant W. side of Cherry Hill opposite 218 Cherry Hill 200' W. on Cherry Hill from Gem. Elev. 947.74.

B.M. 139 Top stem of hydrant W. side of Cherry Hill opposite 1941 Cherry Hill 156' E. on Cherry Hill from Gem. Elev. 938.41.

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil-sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

P.V.I. Sta. 583+00
Elev. 972.99
G₁=2.581% G₂=-0.856%
M.O. = 2.2340



PROFILE ALONG LEFT STRUCTURE

A.D.T. (1975) 44,910 (Each Structure)

PROPOSED STRUCTURE

TYPE: Continuous steel beam with reinforced concrete deck and substructure.
SPANS: 40'-50'-40' c.c. Bearings Left. 35'-50'-35' c.c. Bearings Right
ROADWAY: 42'-0" ¹/₂ parapets with 1'-2" curbs
LOAD FREQUENCY: C.F. 2000 (57) *
SKEW: 14° 40' 01" L.F.
WEARING SURFACE: 1" Monolithic concrete
APPROACH SLABS: AS-1-54 (25' long)
ALIGNMENT: Tangent

* Adequate for alternate A.A.S.H.O. loading.

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**SITE PLAN
BRIDGE NO. MAH-18-1678 L&R
OVER CHERRY HILL AVENUE**

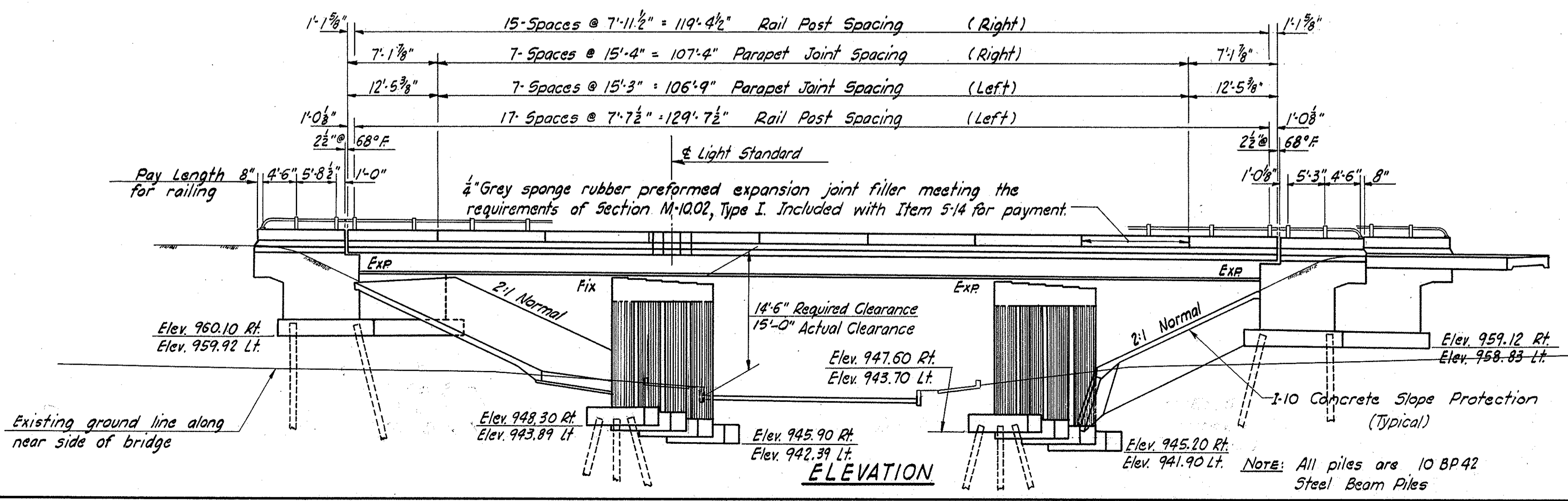
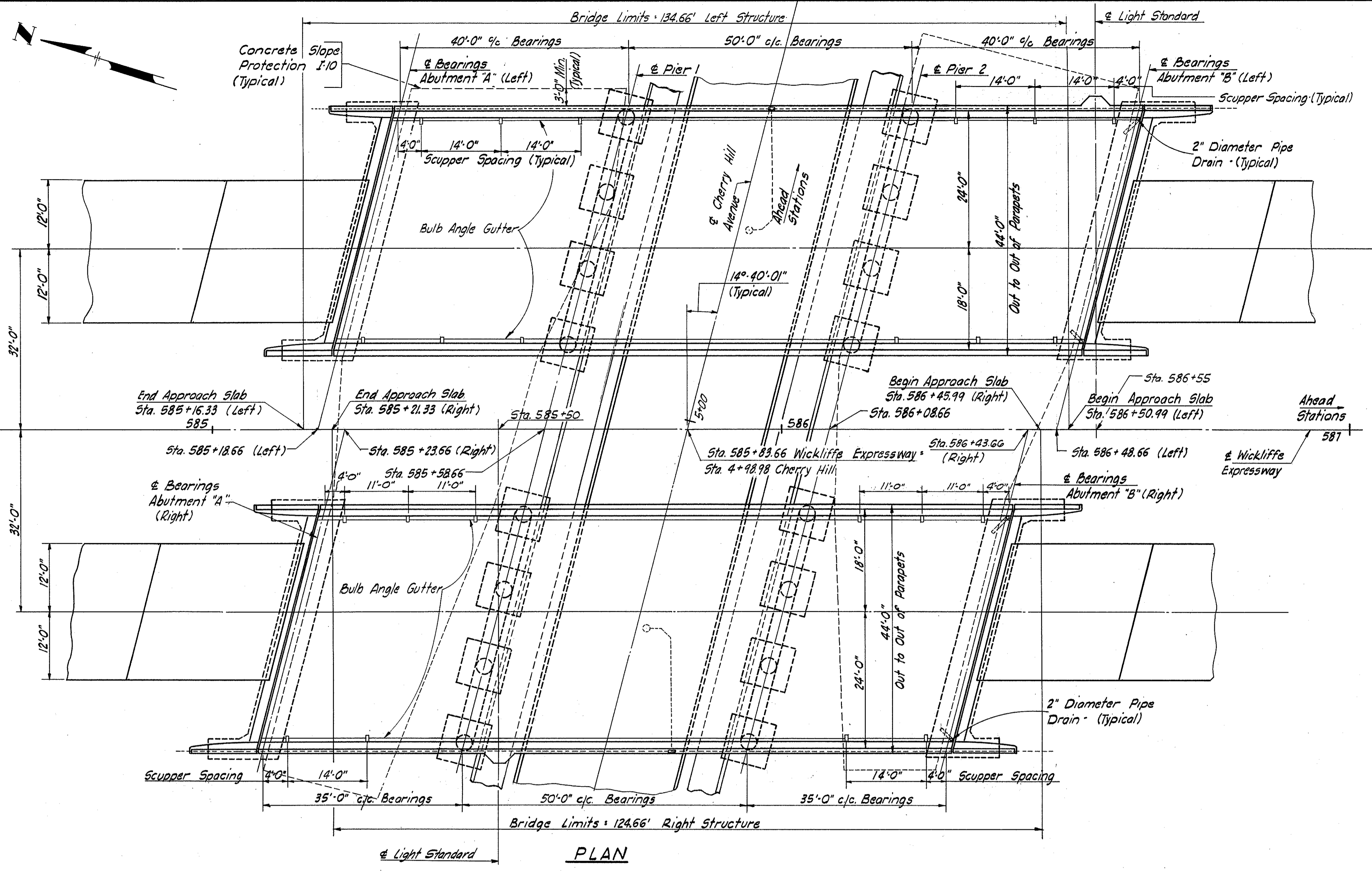
MAHONING COUNTY STA. 585+16.33 L.B. STA. 585+21.33 R.B. STA. 586+50.99 L.B. STA. 586+45.99 R.B.

PRESENT TOPOGRAPHY		PROPOSED WORK			
SURVEYED	DRAWN	DESIGNED	DRAWN	CHECKED	REVIEWED
Aurilio	MM	E.F.R.	J.H.	R.S.	8/16-63

**MAH-18-15.50
MAHONING COUNTY**

GENERAL NOTES:

- **DESIGN SPECIFICATIONS:** This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.
 - **REFERENCE** shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59, AR-1-57 revised 4-2-62, FS-1-62 revised 1-15-63, AS-1-54 revised 7-5-62, and Supplemental Specifications 5-101 dated 12-2-59.
 - **EXCAVATION QUANTITY** includes the removal of fill material required for construction of the abutments.
 - **PILES** shall be driven to a minimum bearing capacity of 26.5 tons at abutments and 31.6 tons at piers.
 - **WELDING** of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.
 - **UTILITY LINES:** All expense involved in relocating the affected utility lines shall be borne by the owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.
 - **MAINTENANCE OF TRAFFIC:** For maintenance of traffic on Cherry Hill Avenue, refer to "General Notes - Traffic."
 - **REFERENCE** shall be made to Supplemental Specification 5-307 dated 8-23-60.
 - **DESIGN LOADING:** CF-2000 (57)
 - **CONCRETE CLASS "C":** Basic unit stress 1,333 psi.
 - **CONCRETE CLASS "E":** Basic unit stress 1,133 psi.
 - **STRUCTURAL STEEL:** ASTM A36 - basic unit stress 20,000 psi. (ASTM A7 and A373 steel not permitted.)
 - **REINFORCING STEEL:** ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade, Basic unit stress 20,000 psi. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 psi.
- REFERENCE shall be made to Standard Drawing 5D-2-64 dated 11-25-64 and Supplemental Specification 5-307 revised 10-1-64.

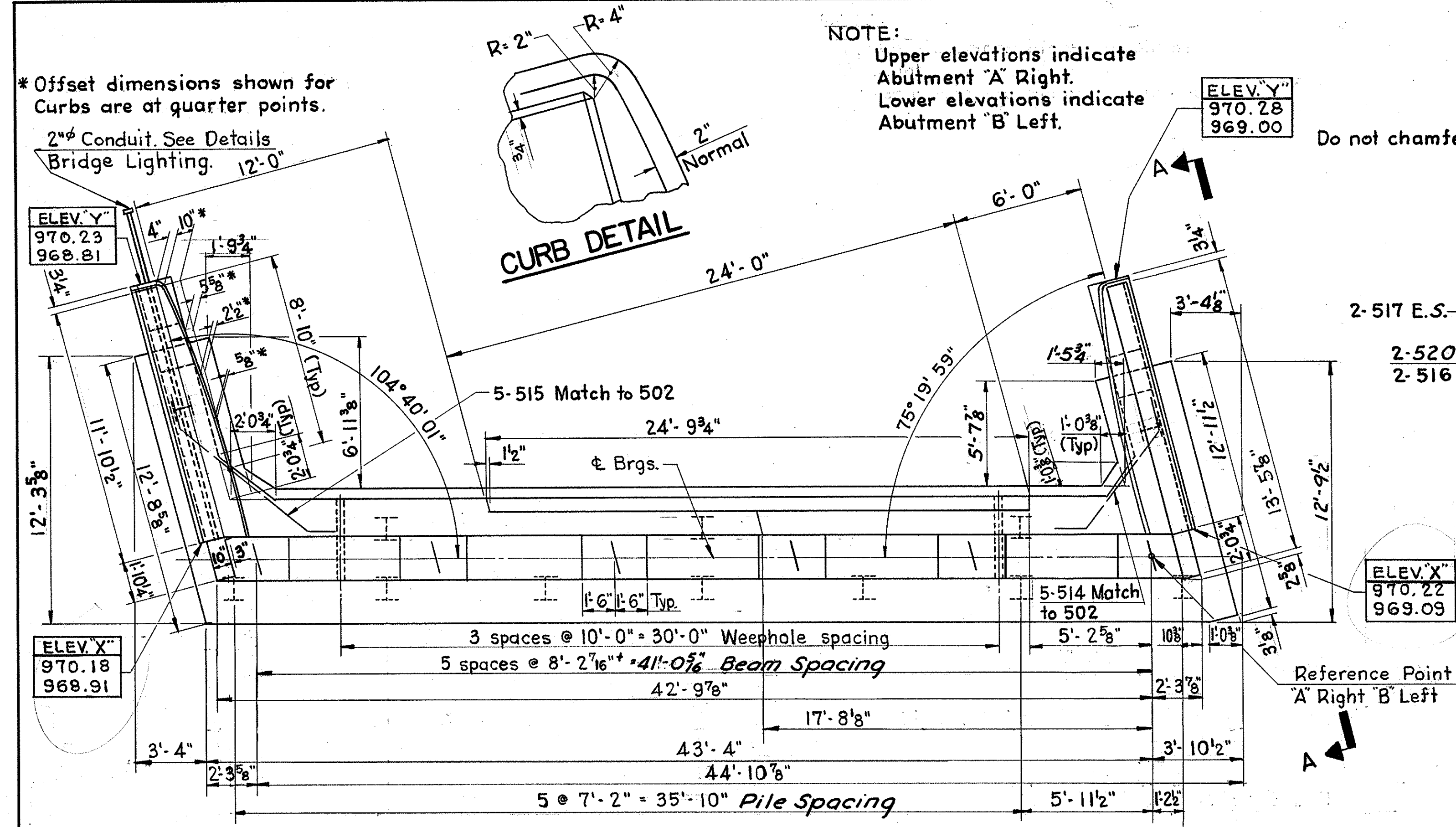


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ROCHESTER, PENNSYLVANIA

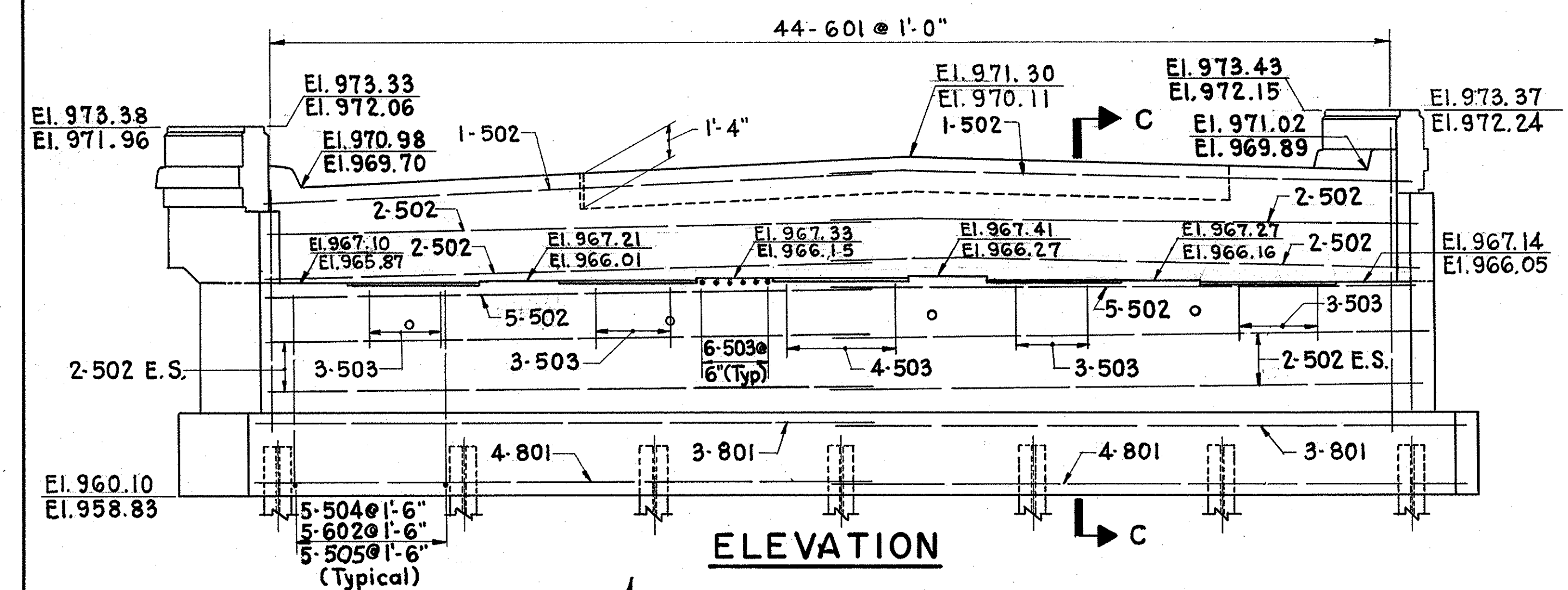
GENERAL PLAN & ELEVATION
BRIDGE NO. MAH-18-1678 L&R
OVER CHERRY HILL AVENUE

MAHONING COUNTY	STA 585+16.33 L.B.	STA 585+21.33 R.B.	STA 586+50.99 L.B.	STA 586+50.99 R.B.
Designed	Drawn	Traced	Checked	Reviewed
EPR	EPR	M.M.	RLS	PKR
				12-16-63

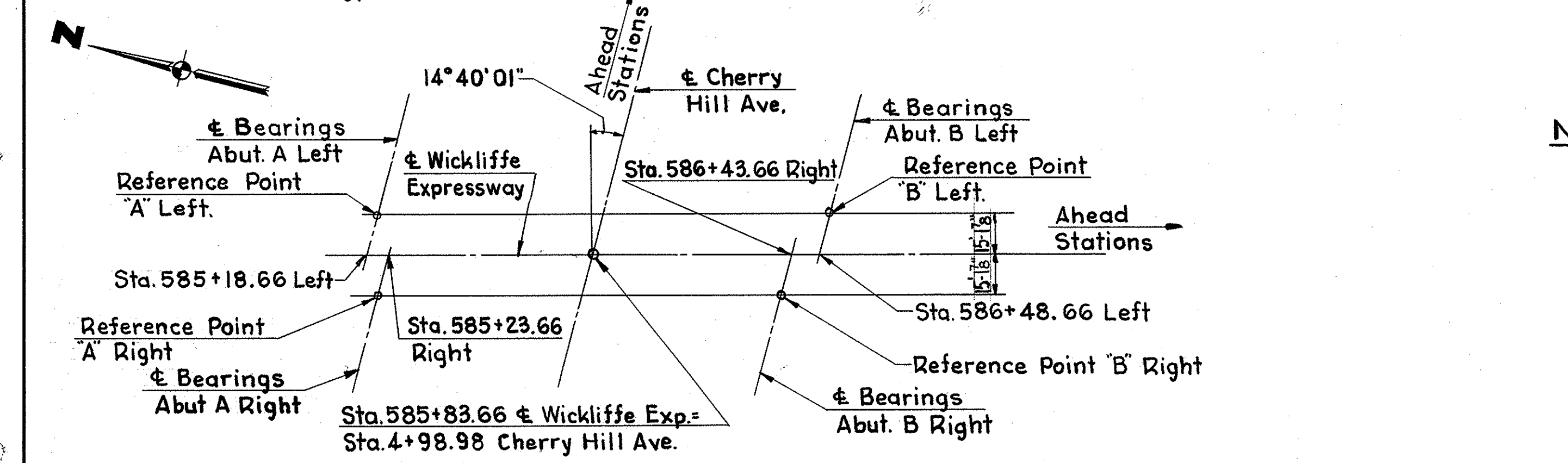
MAH-18-15.50
MAHONING COUNTY



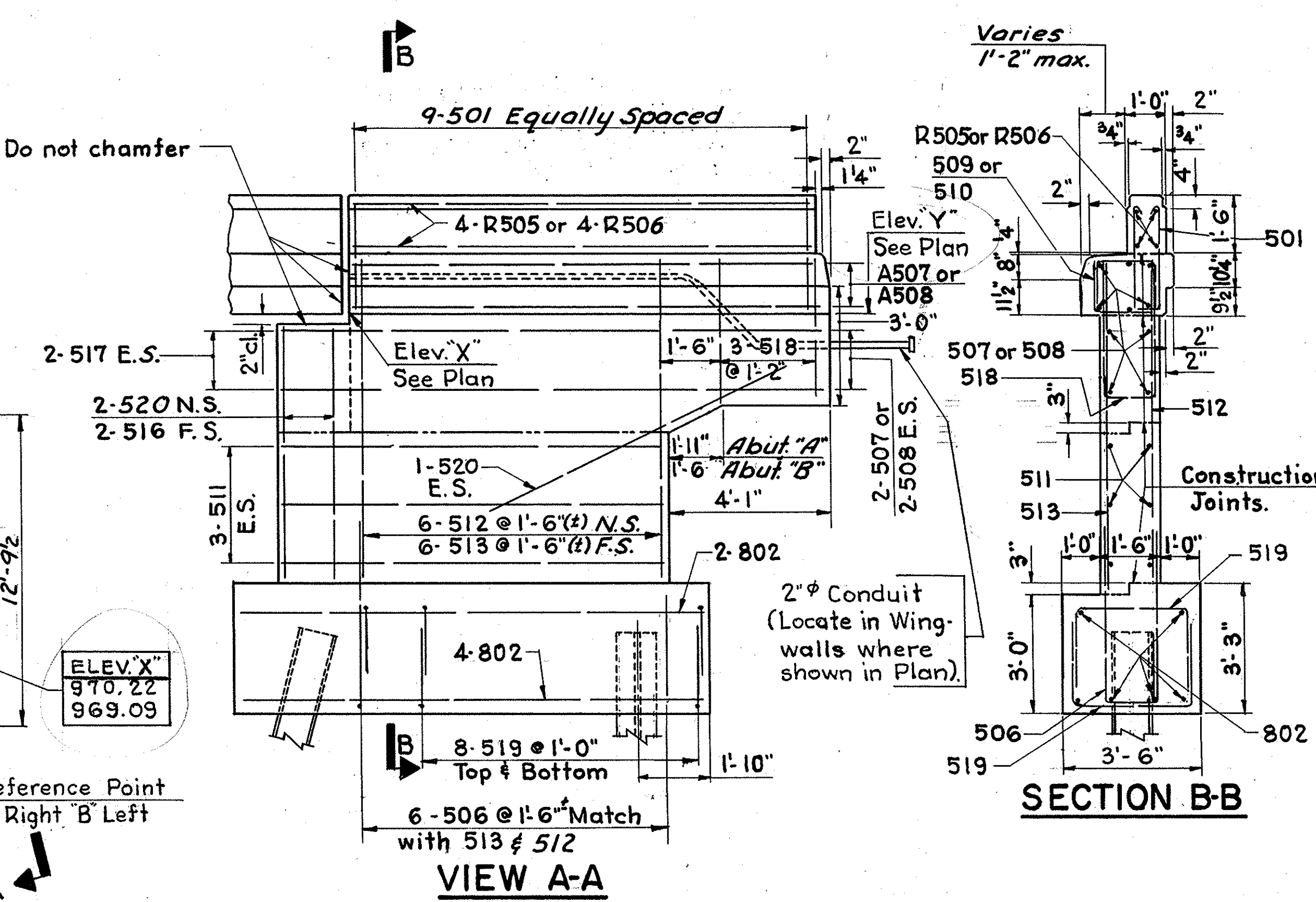
PLAN



ELEVATION

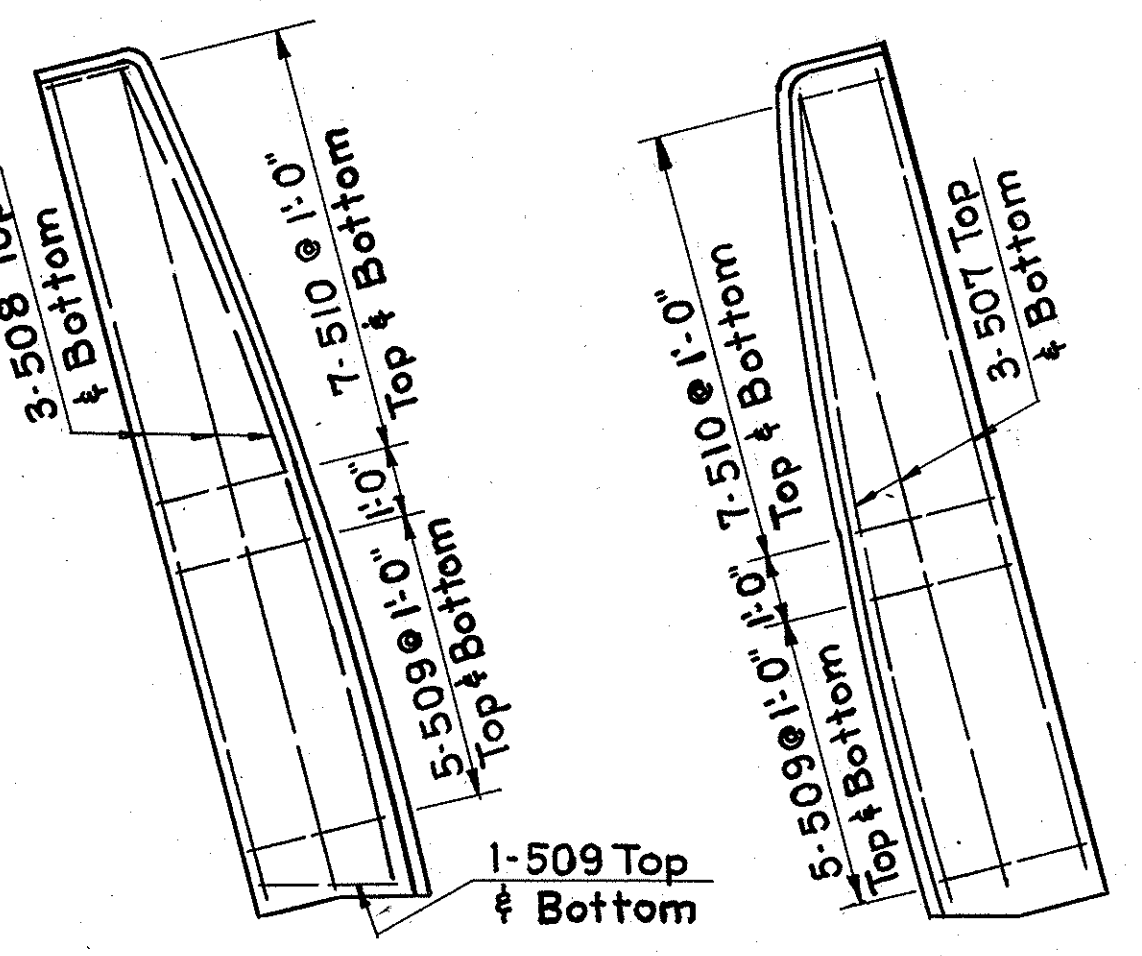


ABUTMENT LAYOUT PLAN

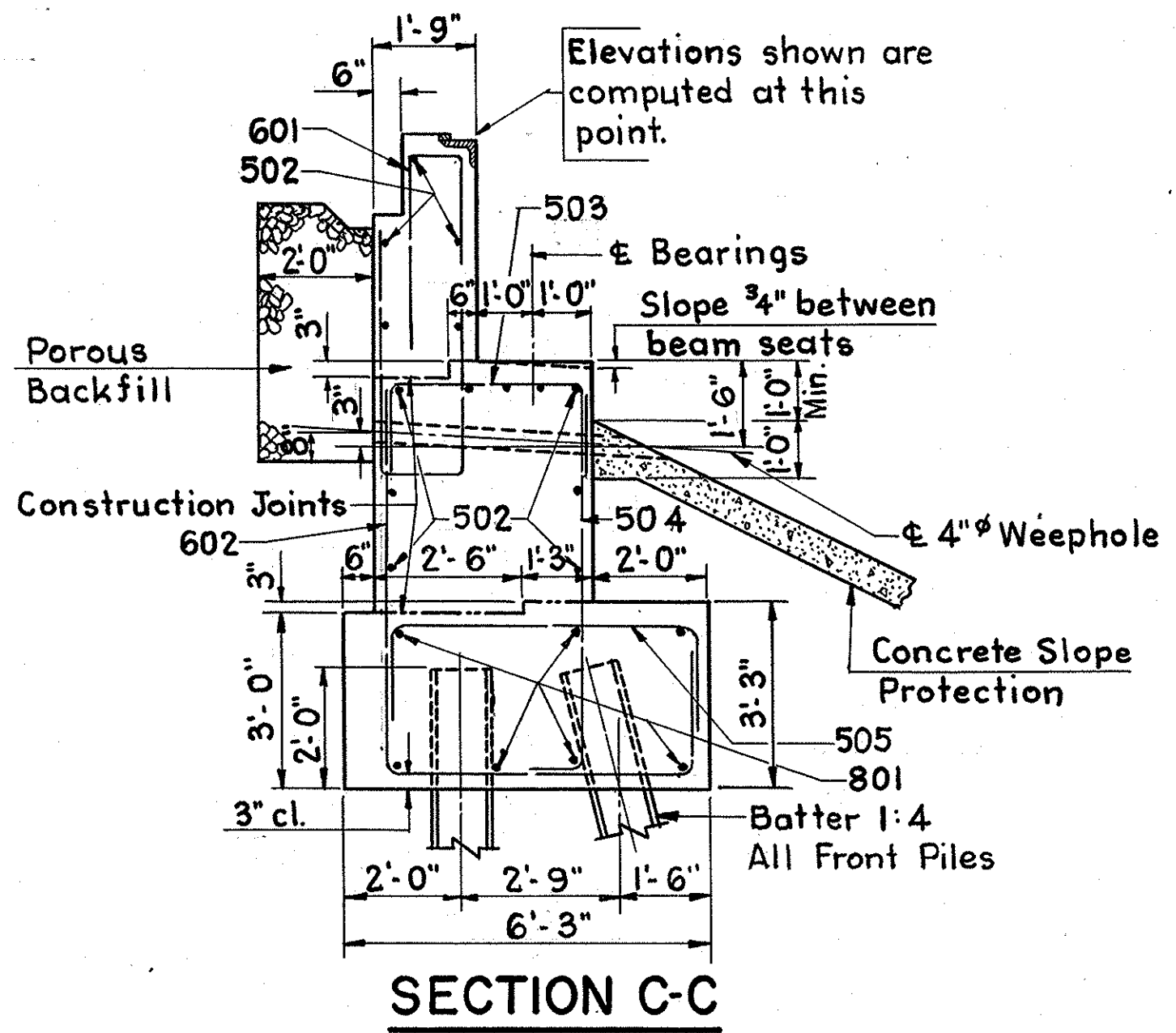


VIEW A-A

SECTION B-B



CURB REINFORCEMENT



SECTION C-C

NOTE:
Bar Marks A are for Abutment A Right.
Bar Marks D are for Abutment B Left.
For Anchor Rod Layout, see Sheet 4

LEGEND
N.S. = Near Side
F.S. = Far Side
E.S. = Each Side

NOTES:

- Porous backfill 2 feet thick full length of abutment shall extend up to the underside of approach slab and outward to the wings.
- All abutment concrete shall be Class "E".
- All parapet concrete shall be Class "C".
- Concrete and reinforcing steel above parapet construction joint shall be included with railing S-14 for payment.
- All abutment parapet railings shall be continuous through railing posts.
- Refer to Standard Drawing AR-1-57 (Revised 4-2-62) for details of Type "A" railing and concrete parapet.
- PROCEDURE: The embankment shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade for a distance of 200 feet back of the abutments, after which excavation shall be made and piles driven for the abutments and piers.
- BRIDGE SEAT REINFORCING: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.

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ROCHESTER, PENNSYLVANIA

ABUTMENTS "A" RT., "B" LT.
BRIDGE NO. MAH-18-1678 L.&R.
OVER CHERRY HILL AVENUE

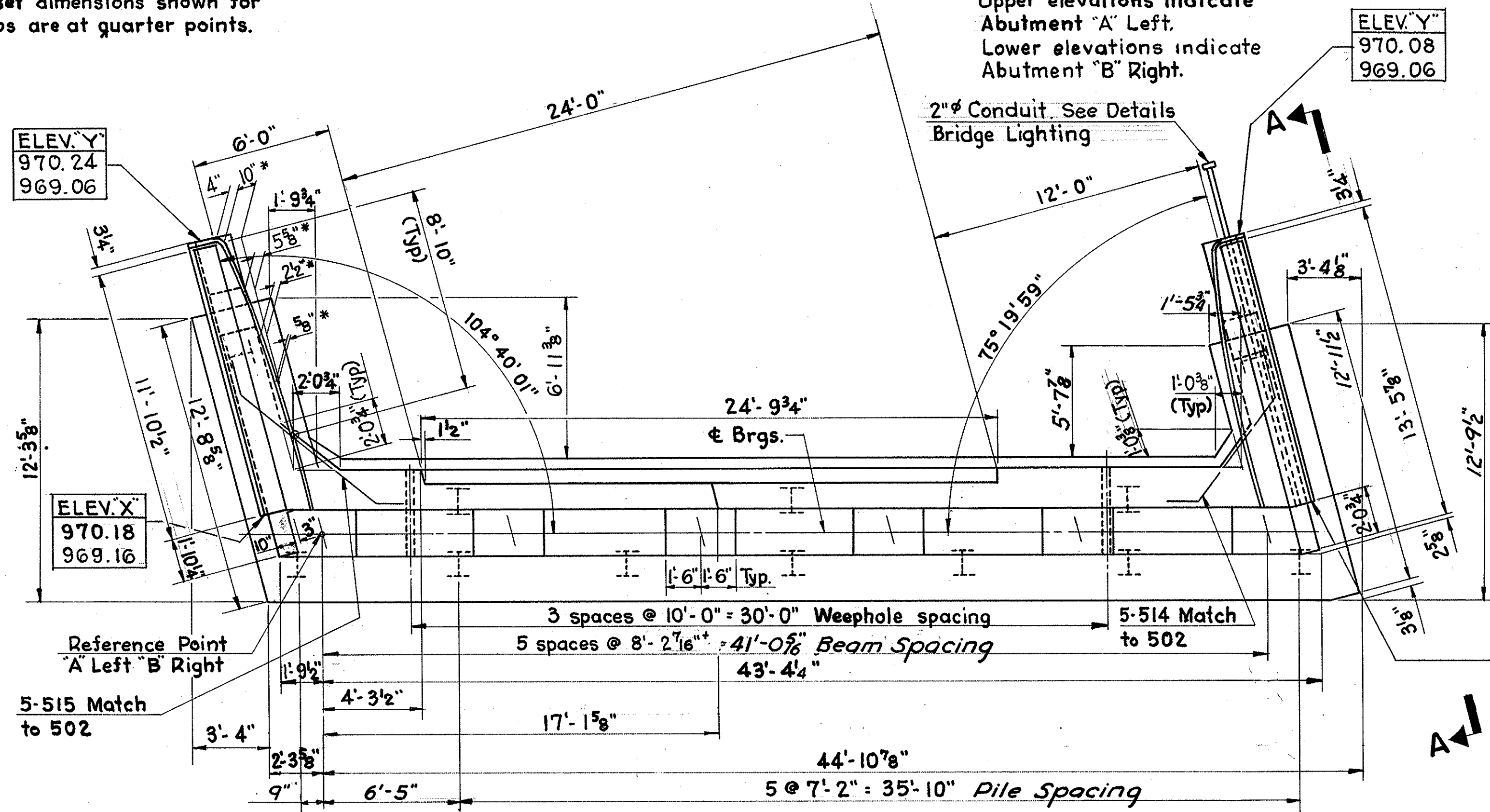
MAHONING COUNTY STA. 585+16.33 L.B., STA. 585+21.33 R.B.
STA. 586+50.99 L.B., STA. 586+45.99 R.B.

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
EFR	EFR	P.T.R.	KAE	DK	8-16-63	

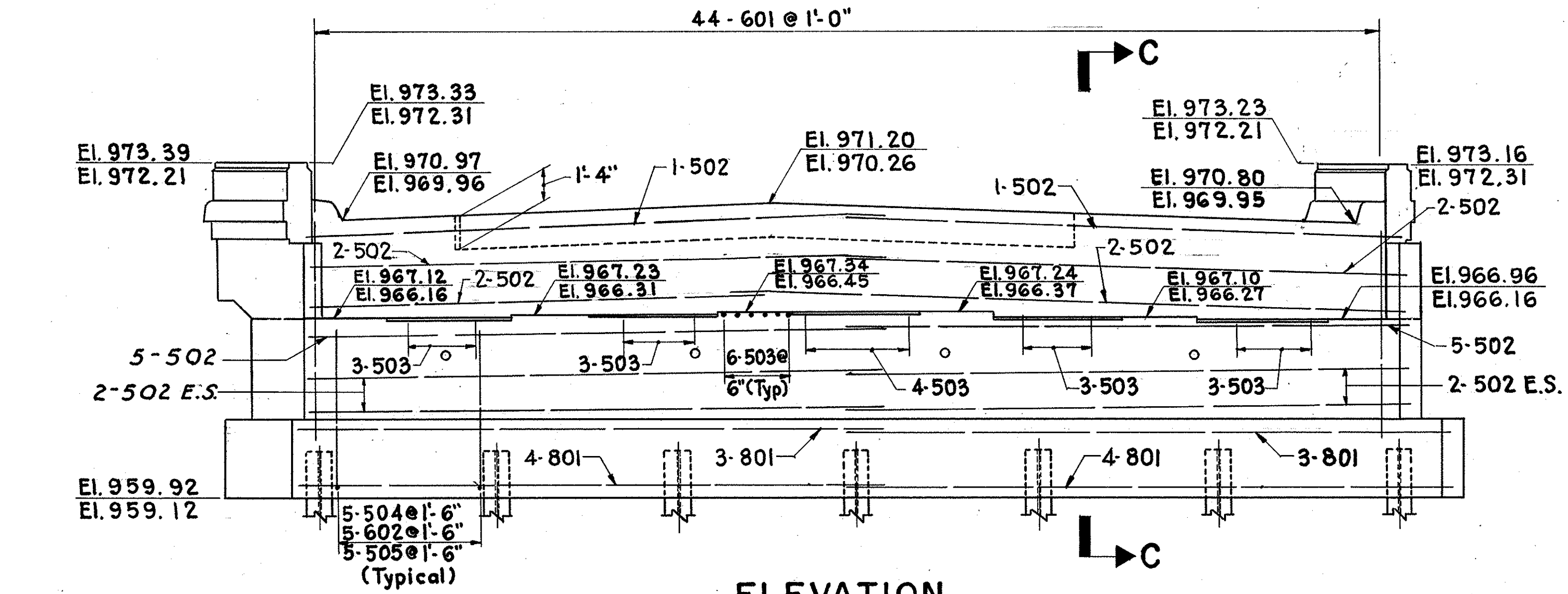
MAH-18-15.50
MAHONING COUNTY

* Offset dimensions shown for Curbs are at quarter points.

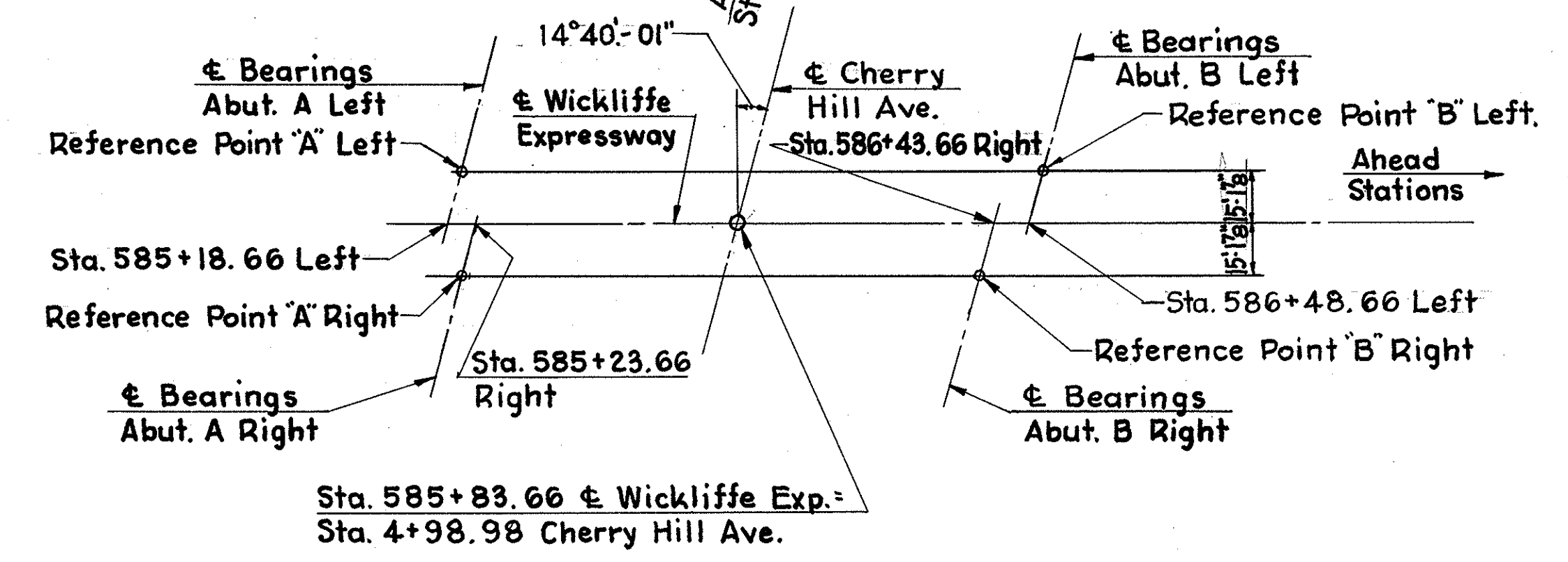
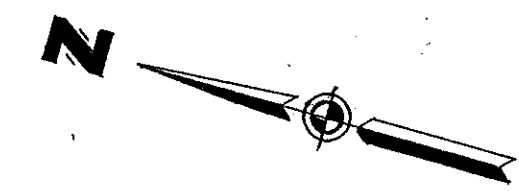
NOTE:
Upper elevations indicate Abutment "A" Left.
Lower elevations indicate Abutment "B" Right.



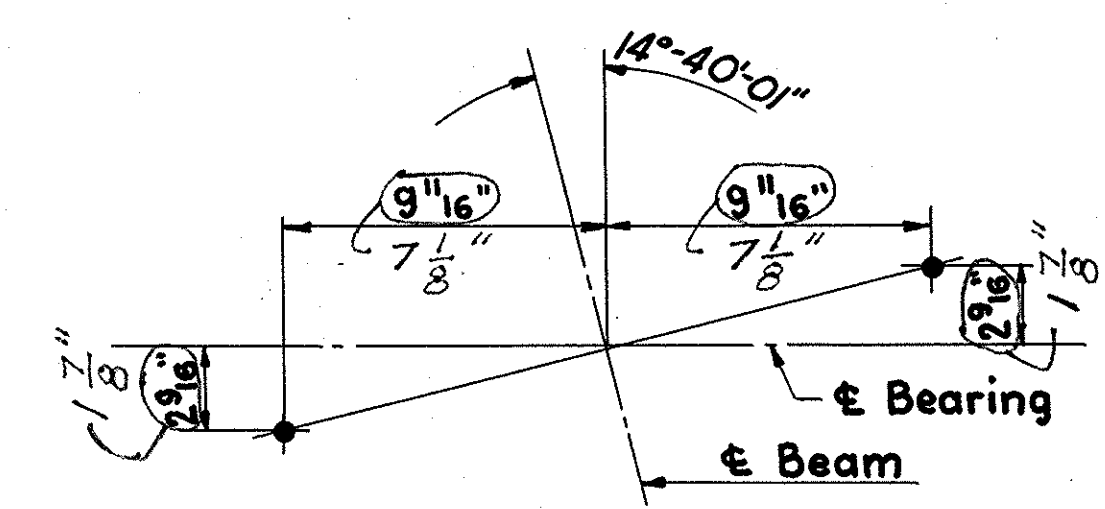
PLAN



ELEVATION



ABUTMENT LAYOUT PLAN



ANCHOR ROD LAYOUT DIAGRAM

NOTE:
For Curb Details, View A-A, Section B-B, Section C-C, Curb Reinforcement and Notes see Sheet No. 3.

NOTE:
Bar Marks B ___ are for Abutment B Right.
Bar Marks C ___ are for Abutment A Left.

LEGEND
N. S. = Near Side
F. S. = Far Side
E. S. = Each Side

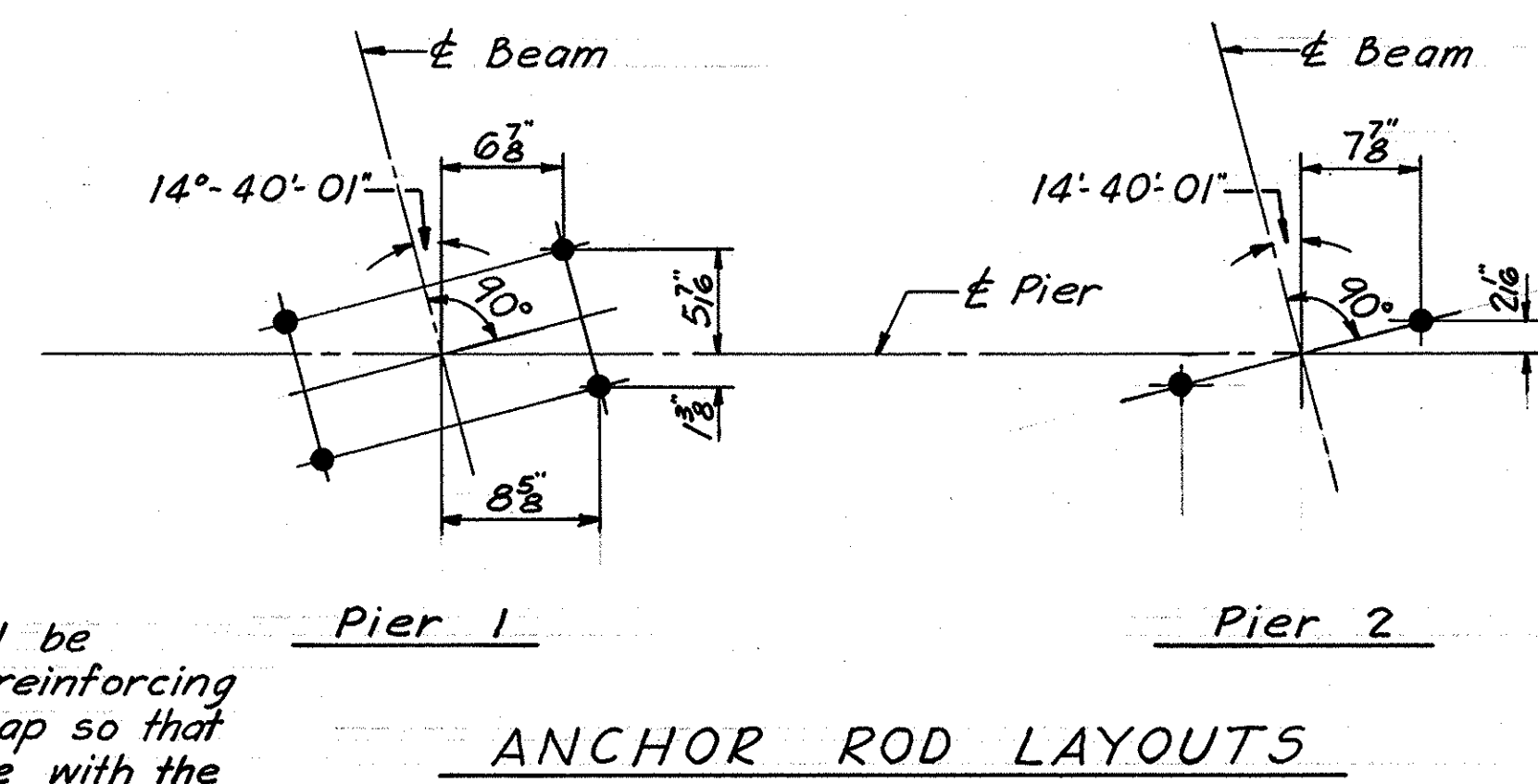
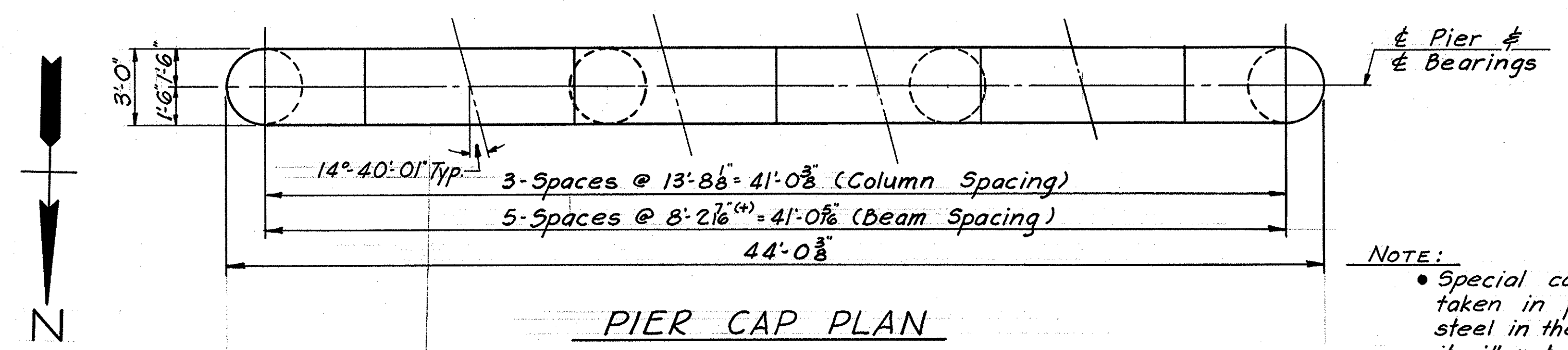
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ROCHESTER, PENNSYLVANIA

ABUTMENTS "A" LT., "B" RT.
BRIDGE NO. MAH-18-1678 L.&R.
OVER CHERRY HILL AVENUE

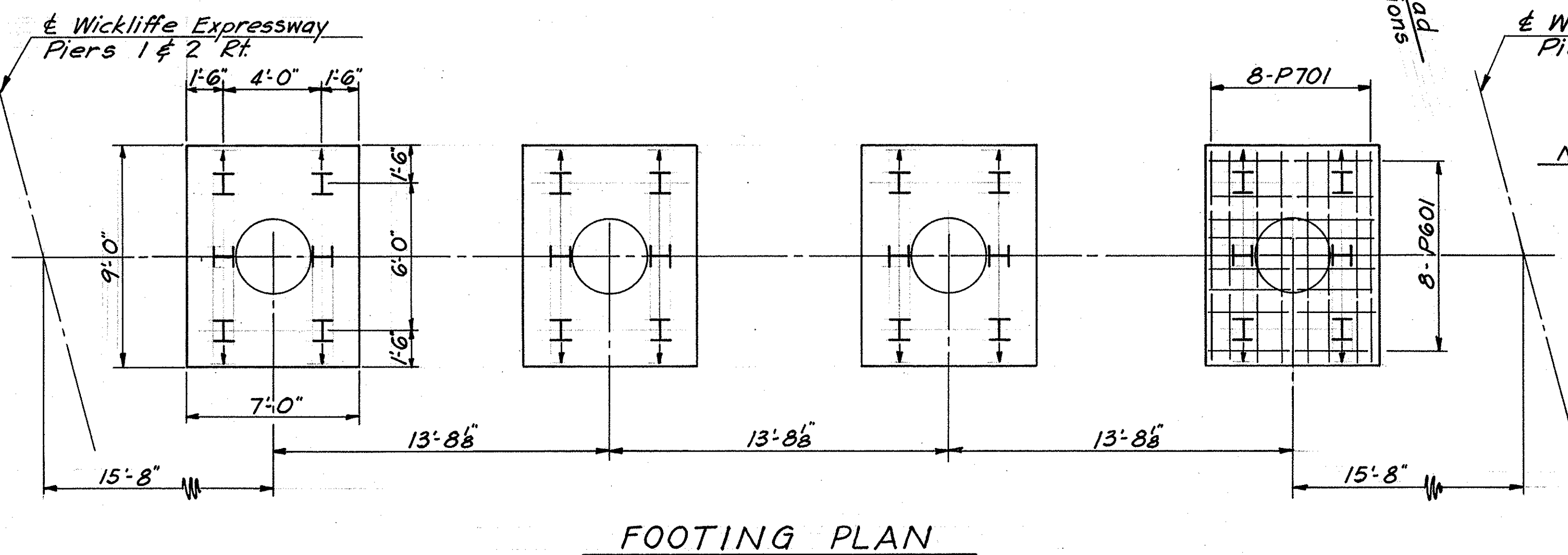
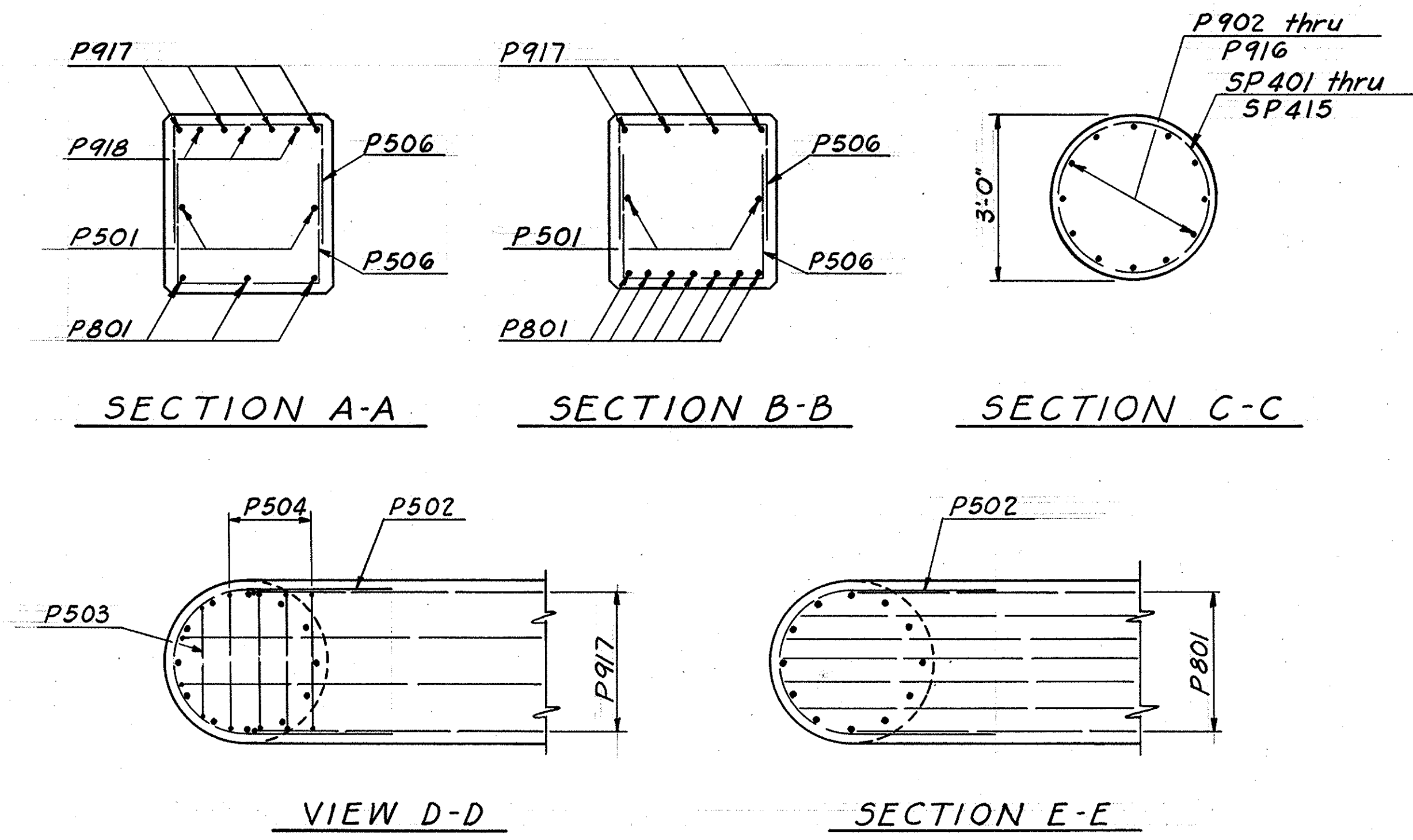
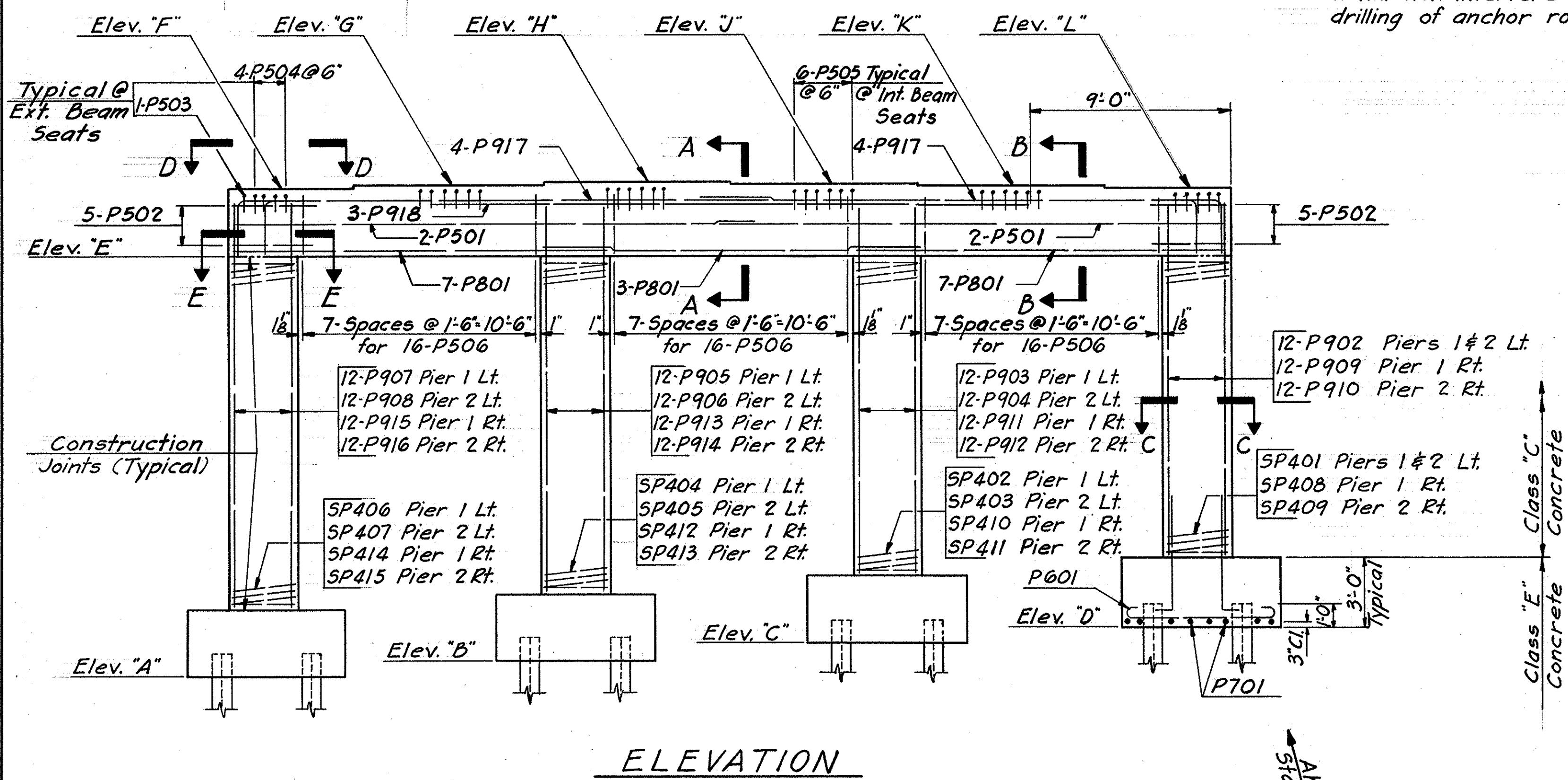
MAHONING COUNTY STA. 585+16.33 L.B. STA. 585+21.33 R.B.
STA. 586+50.99 L.B. STA. 586+45.99 R.B.

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
EPR	EPR	P.T.R.	K.A.E.	DFK	3-16-63	1-20-26

**MAH-18-15.50
MAHONING COUNTY**



NOTE:
• Special care shall be taken in placing reinforcing steel in the pier cap so that it will not interfere with the drilling of anchor rod holes



NOTE:
• Piles shown thus ↑ are to be battered 1:4 in direction indicated by arrow.

NOTE:
• Super-Structure Ground-A No. 1/0 AWG 7 strand soft annealed bare copper cable shall be encased in the outside column of pier No. 1. Connect (by Exothermic weld process) lower end of cable to a steel pile. Extend cable (in one continuous length) through top of pier with lead of sufficient length to exothermic weld upper end of cable to outside beam of super-structure
• Payment for electrical grounds is included in the lump sum bid for 5-25 "Electrical Lighting System Complete"

Location	Elev. 'A'	Elev. 'B'	Elev. 'C'	Elev. 'D'	Elev. 'E'	Elev. 'F'	Elev. 'G'	Elev. 'H'	Elev. 'J'	Elev. 'K'	Elev. 'L'
Lt. Bridge Pier 1	942.39	942.89	943.39	943.89	963.39	966.39	966.53	966.67	966.79	966.68	966.57
Lt. Bridge Pier 2	941.90	942.30	943.00	943.70	963.17	966.17	966.31	966.46	966.57	966.46	966.35
Rt. Bridge Pier 1	945.90	946.60	947.50	948.30	963.67	966.68	966.82	966.96	966.89	966.78	966.67
Rt. Bridge Pier 2	945.20	946.00	946.80	947.60	963.46	966.46	966.57	966.67	966.75	966.61	966.46

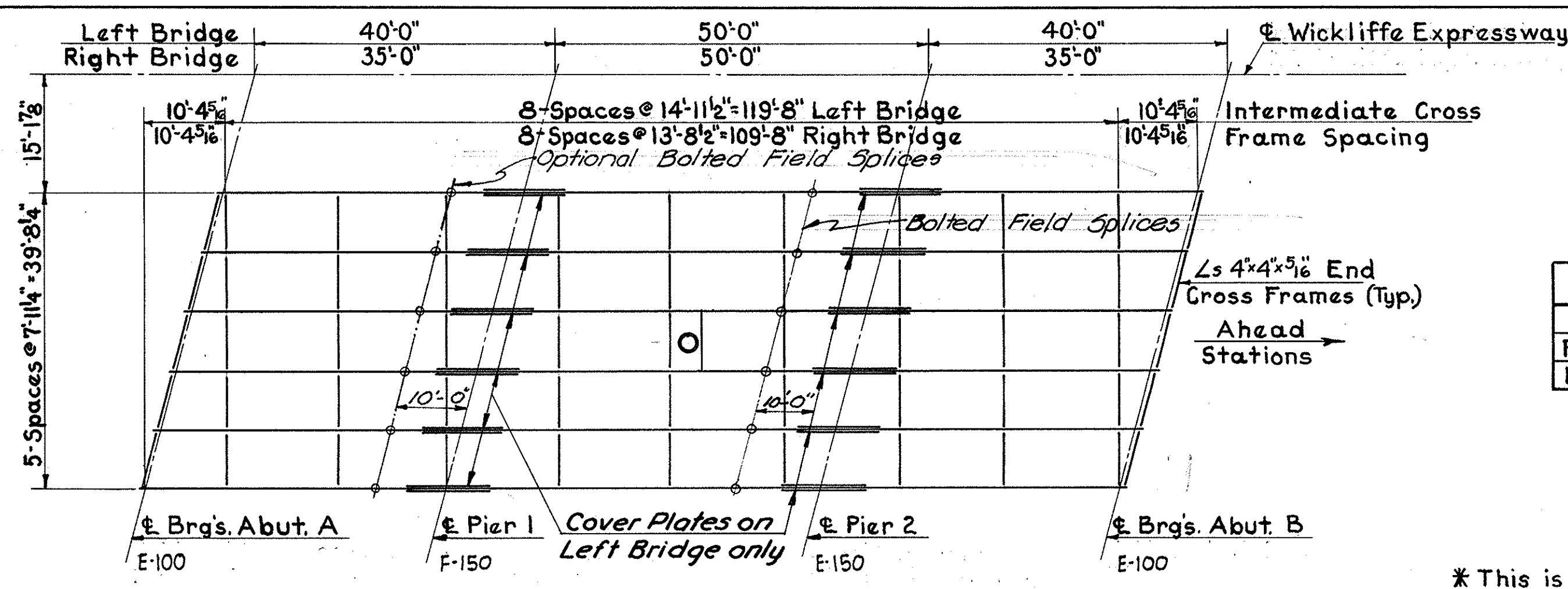
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

PIERS
BRIDGE NO. MAH-18-1678 L.&R.
OVER CHERRY HILL AVENUE

MAHONING COUNTY STA. 585+16.33 L.B. STA. 585+21.33 R.B.
STA. 586+50.99 L.B. STA. 586+45.99 R.B.

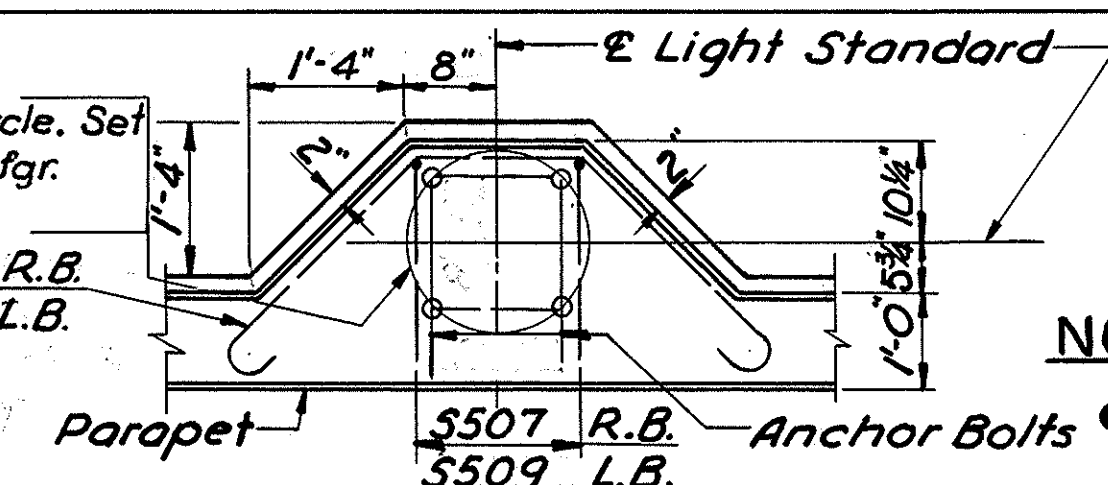
Designed	Drawn	Traced	Checked	Reviewed Date	Revised
RLS	R.G.	RC	JK	8-16-63	1-20-66

**MAH-18-15.50
MAHONING COUNTY**



STEEL FRAMING PLAN

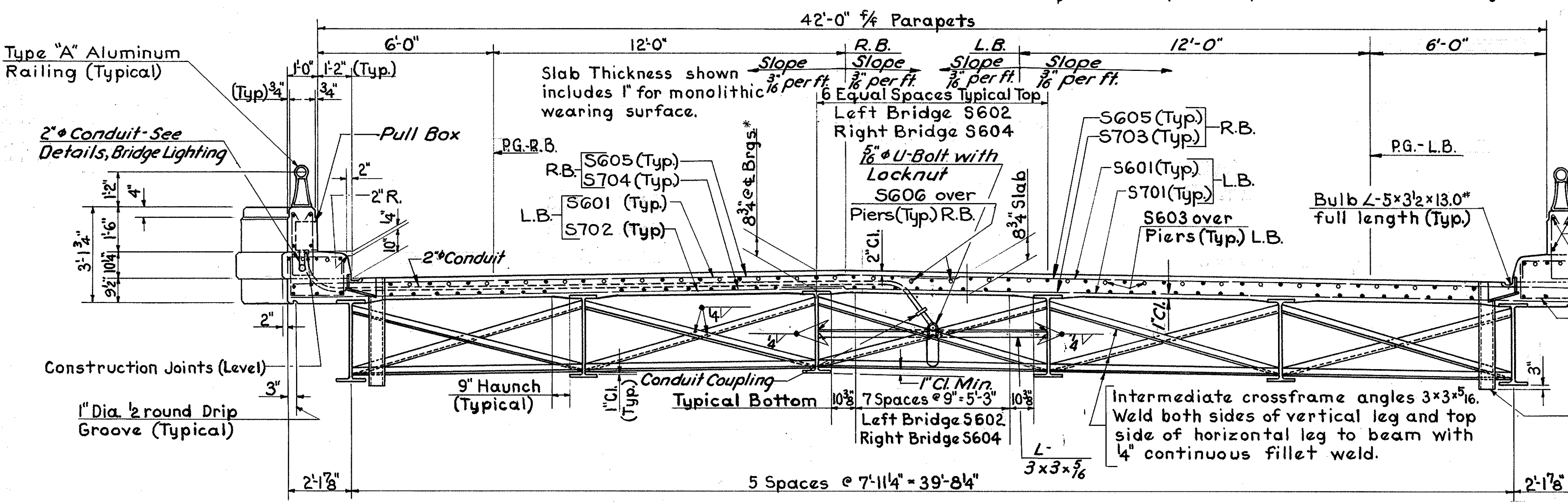
BEAMS			
	Span 1	Span 2	Span 3
Right Bridge	33WF118	33WF118	33WF118
Left Bridge	33WF118	33WF118	33WF118



LIGHT STANDARD DETAIL

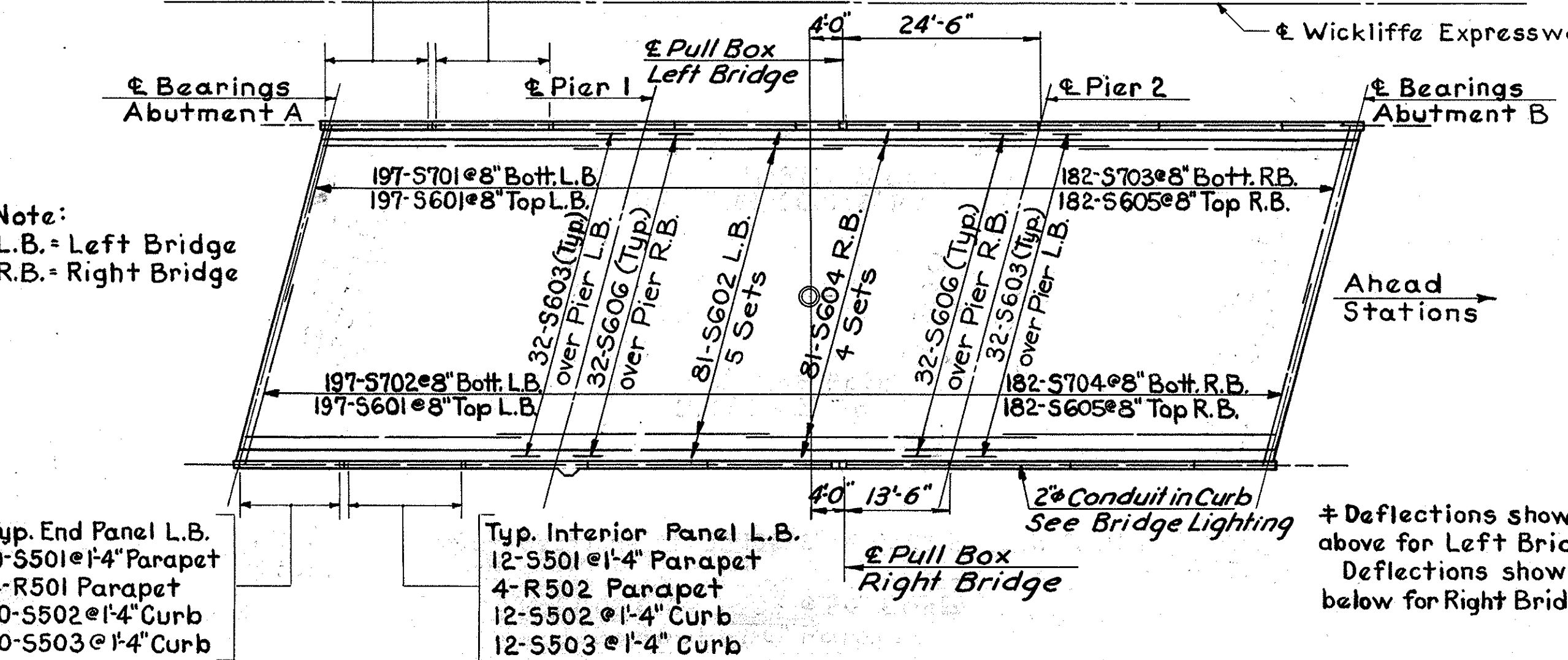
NOTES:

- Refer to Standard Drawing AR-1-57 for details of Type "A" aluminum railing and concrete parapet.
- Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
- See General Plan and Elevation for location of scuppers, 2" pipe drains and railing posts.
- Refer to Standard Drawing CSB-2-56, Sheets 2 of 3 of 6 for details of expansion dams, end crossframes, 2" pipe drains, curb plates and scuppers.
- CONCRETE DECK PLACING:** In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress up grade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
- DECK SLAB HAUNCH:** The haunch in the deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width.
- MACHINE FINISH:** The concrete bridge deck shall be finished by the use of a finishing machine.
- ELECTRICAL LIGHTING SYSTEM:** For additional details, refer to drawings, "Details-Bridge Lighting" and "General Lighting Notes."
- All concrete shall be class "C"

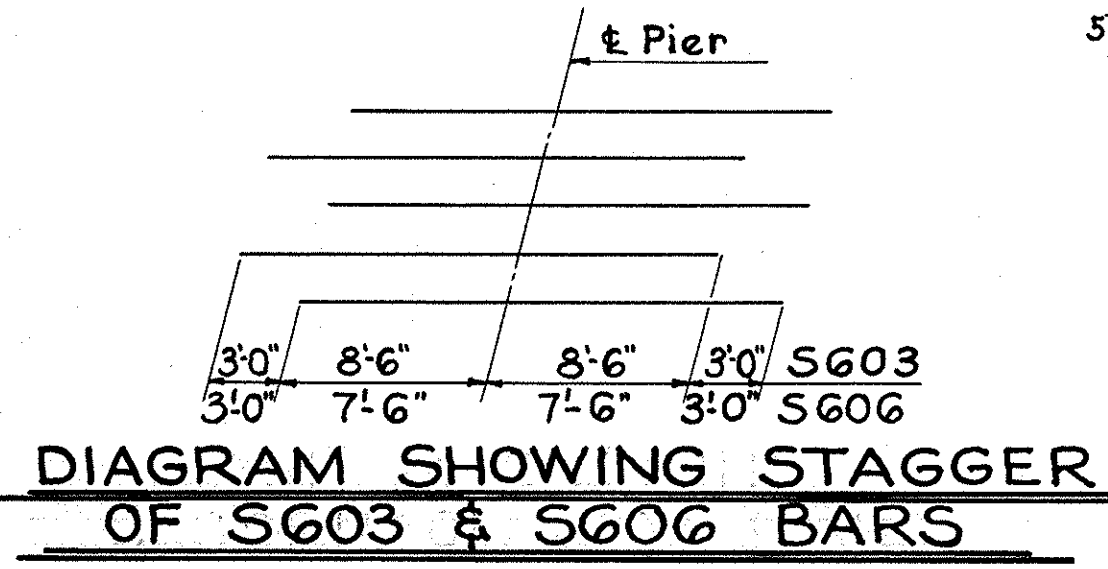


TYPICAL CROSS SECTION

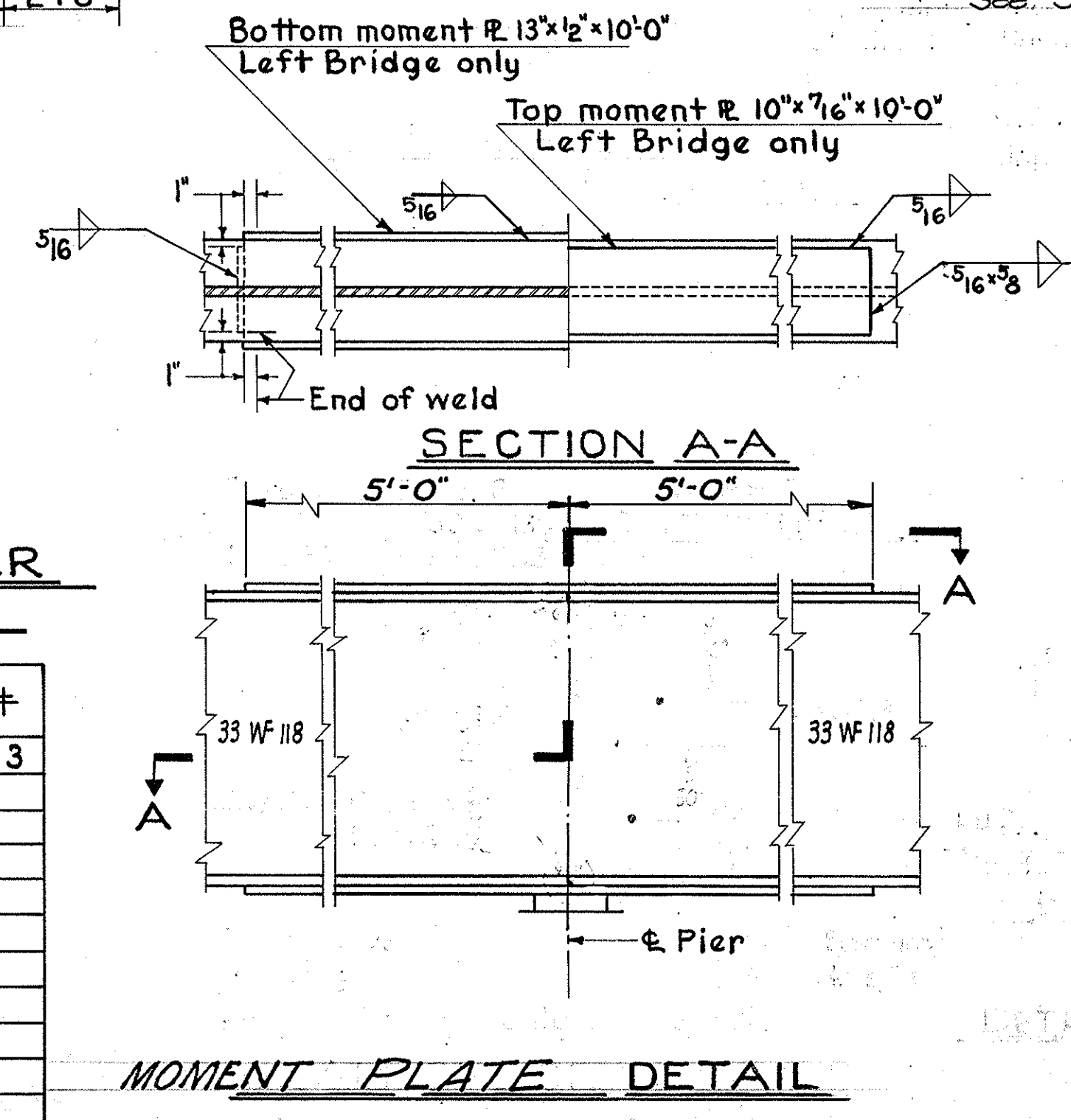
- Typ. End Panel R.B.
6-S504 @ 1'-4" Parapet
4-R503 Parapet
6-S505 @ 1'-4" Curb
6-S506 @ 1'-4" Curb
- Typ. Interior Panel R.B.
12-S504 @ 1'-4" Parapet
4-R504 Parapet
12-S505 @ 1'-4" Curb
12-S506 @ 1'-4" Curb



DECK REINFORCING PLAN



DEFLECTIONS AND CAMBER ±			
LOCATION	SPAN 1	SPAN 2	SPAN 3
Deflection due to weight of steel	0"	0"	0"
Deflection due to remaining D.L.	1/16"	3/16"	1/8"
Convexity required for vertical curve	3/16"	0"	0"
Sum of deflections and convexity	3/16"	3/16"	1/8"
Required Camber	0"	0"	0"



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SUPERSTRUCTURE DETAILS
BRIDGE NO. MAH-18-1678 L&R
OVER CHERRY HILL AVENUE

MAHONING COUNTY STA. 585+16.33 L.B. STA. 585+21.33 R.B.
STA. 586+50.99 L.B. STA. 586+45.99 R.B.

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
EFR	EFR	JH	RLS	JK	8-16-13	

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

261

MAH-18-15.50
MAHONING COUNTY

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil-sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

BENCH MARK 200: Top stem of hydrant on north side of Concord Avenue, 180 feet east from Gem Avenue. Elev. 948.204.

BENCH MARK 201: Top stem of hydrant on north side of Concord Avenue, 430 feet west from Gem Avenue. Elev. 965.264.

A.D.T. (1975) 44,910 (Each Structure)

PROPOSED STRUCTURE

TYPE: 3-Span continuous steel beam with reinforced concrete deck and sub-structure.

SPANS: 56'-0", 70'-0", 56'-0"

ROADWAY: Right 50'-0" $\frac{1}{2}$ parapets including 1'-2" curbs. Left varies.

LOAD FREQUENCY: CF 2000 (57) (Adequate for alternate A.A.S.H.O. Loading).

SKWEAR: 22° 58' L.F.

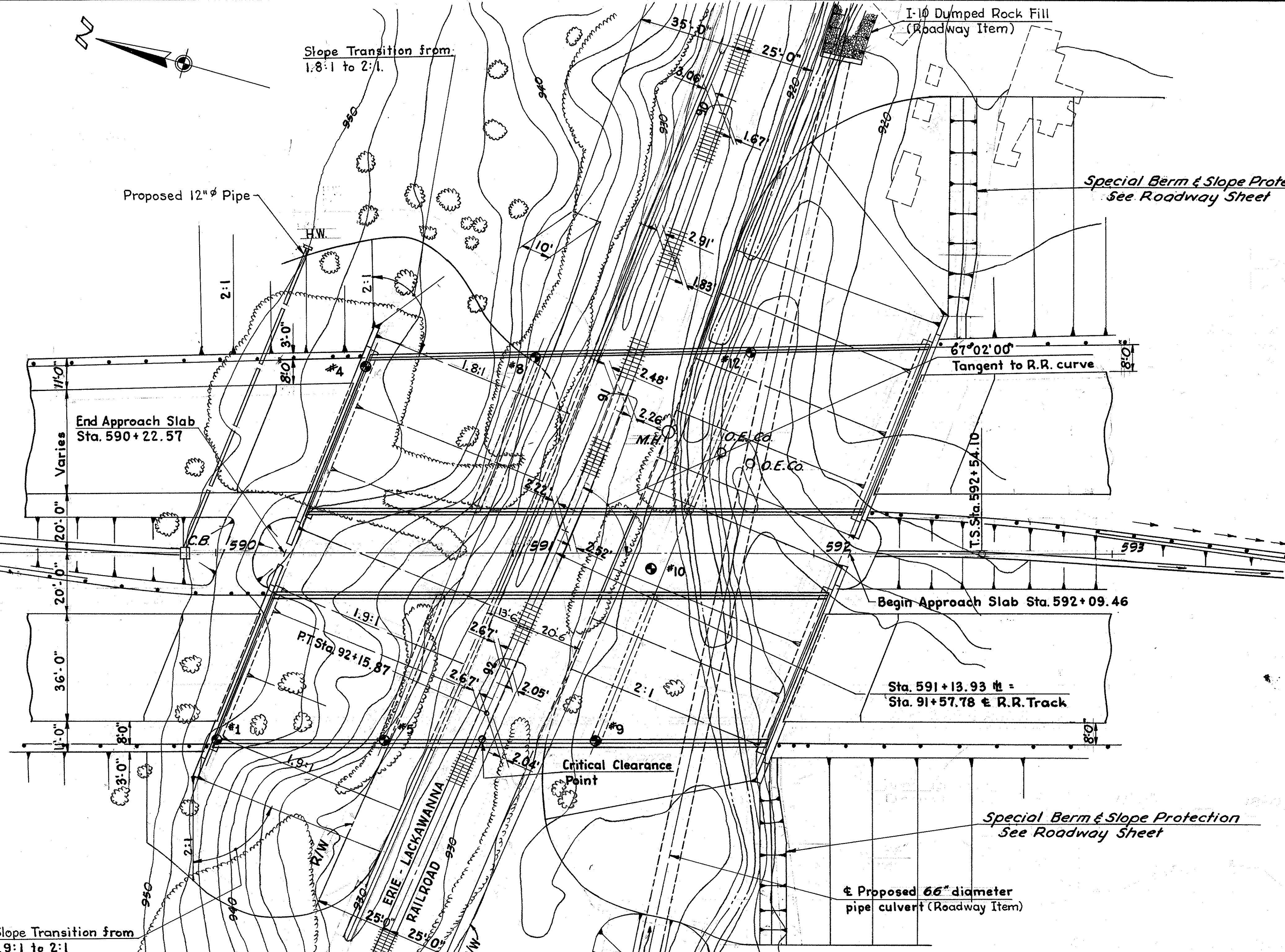
WEARING SURFACE: 1" Monolithic concrete.

APPROACH SLABS: AS-1-54 (25' long).

ALIGNMENT: Tangent.

€ R.R. RIGHT OF WAY

P.I. Sta. 90+83.37
 $\Delta = 07^\circ 30'$
 $D_c = 02^\circ 50' 01''$
 $T = 132.54'$
 $L = 264.68$
 $R = 2022.17$
 P.C. Sta. 89+51.19
 P.T. Sta. 92+15.87

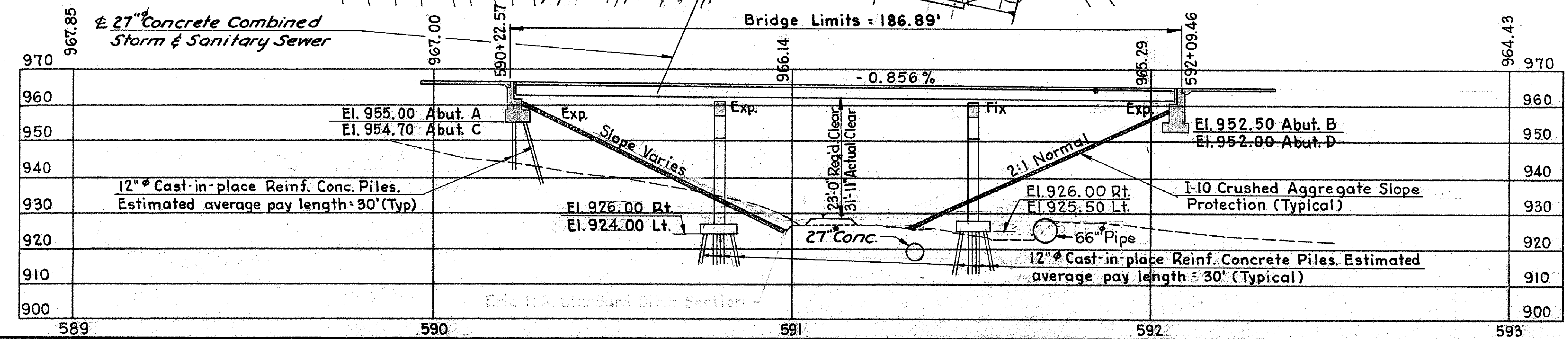


€ Wickliffe Expressway

⊙ Indicates boring location.

P.V.I. Sta. 583+00
 Elev. 972.99
 $G_1 = +2.58\%$
 $G_2 = -0.856\%$

Slope Transition from 1.9:1 to 2:1



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SITE PLAN

BRIDGE NO. MAH-18-1688 L.&R.
 OVER ERIE - LACKAWANNA R.R.

STA. 590+22.57
 STA. 592+09.46

PRESENT TOPOGRAPHY			PROPOSED WORK		
SURVEYED	DRAWN	DESIGNED	DRAWN	CHECKED	REVIEWED
Aurilio	CWC	BM	P.T.R.	V.R.	BM

**MAH-18-15.50
MAHONING COUNTY**

GENERAL NOTES:

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.

REFERENCE shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59; AR-1-57 revised 4-2-62; FSB-1-62 revised 1-15-63; Supplemental Specifications S-101 dated 7-12-62 and AS-1-54 revised 7-5-62.

EXCAVATION QUANTITY includes the removal of fill material required for construction of the abutments and piers.

PILES shall be driven to a minimum bearing capacity of 31.3 tons per pile for the abutments and 39.1 tons per pile for the piers.

FOUNDATION BEARING PRESSURE: Abutment B and D footings are designed for a maximum bearing pressure of 1.8 tons per square foot.

WELDING of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.

RAILROAD AERIAL LINES will be relocated by the railroad. The Contractor shall use all precautions necessary to see that the lines are not disturbed during the construction stage and shall cooperate with the railroad in the relocation of these lines. The cost of the relocation shall be included in the railroad force account work.

CONSTRUCTION CLEARANCE of 27'-0" vertically above the top of the railroad rails and 8'-0" horizontally from the center of tracks shall be maintained at all times.

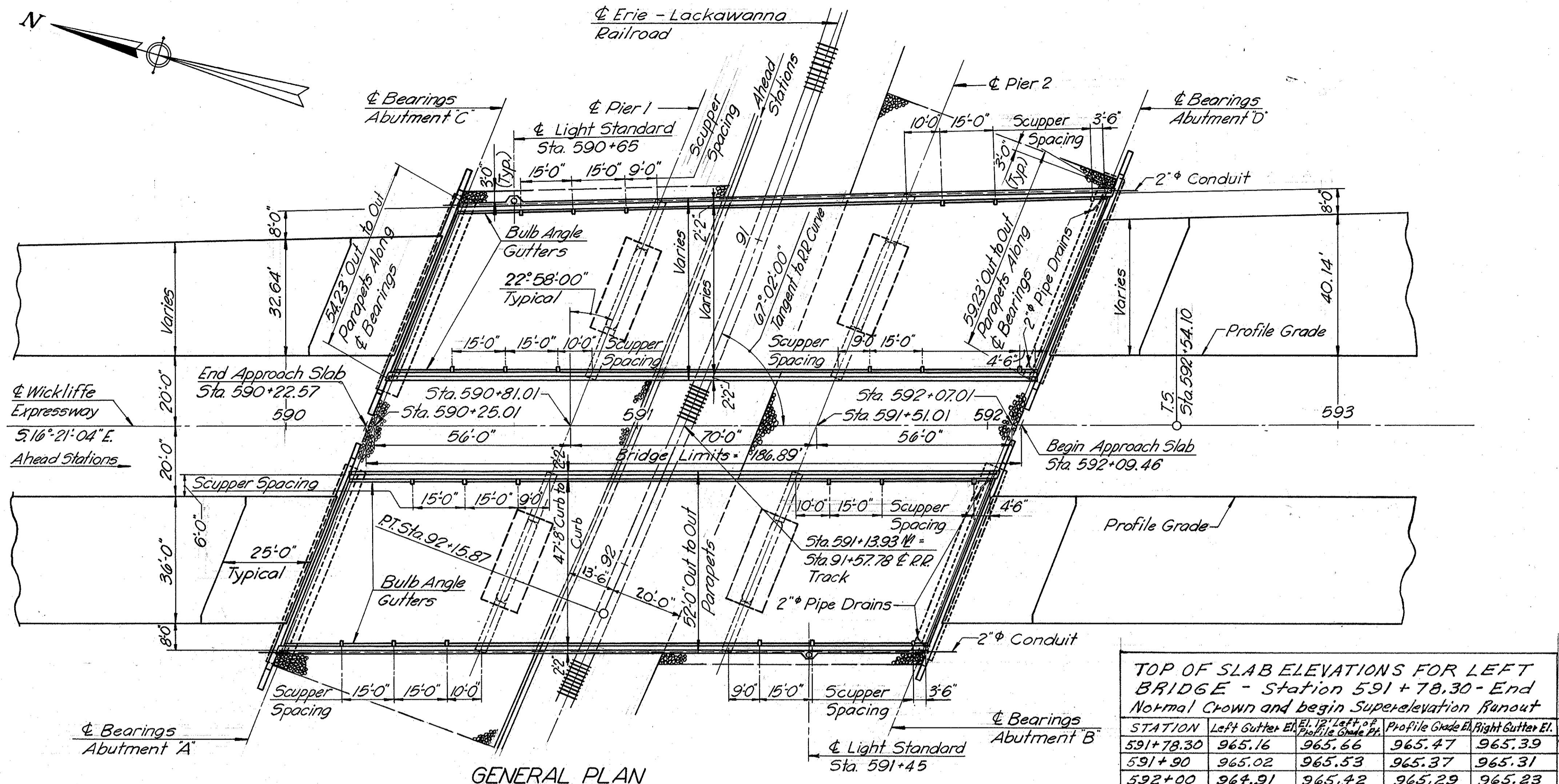
SHEETING AND BRACING: Before construction is started, eight sets of prints showing details of the sheeting and bracing to be used for excavation adjacent to the railroad tracks shall be submitted to the Director for approval by the Department of Highways and by the railroad company.

ALIGNING RAILROAD TRACKS: After the Contractor has completed all excavation and backfill adjacent to the railroad tracks in compliance with Sec. E-2.04 and E-2.08 of the Construction and Material Specifications, subject to the supervision of the railroad company, nothing in Sec. E-2.04, E-2.08 or G-8.07 of the Specifications shall be construed to hold the Contractor liable for aligning and resurfacing the railroad tracks.

EMBANKMENT PROCEDURES: The embankment shall be placed and compacted up to the finished Spill-thru Slopes and to the level of the Subgrade for a distance of 200 feet back of the abutments, after which excavation shall be made for the abutments and for piers that are set in the filled area.

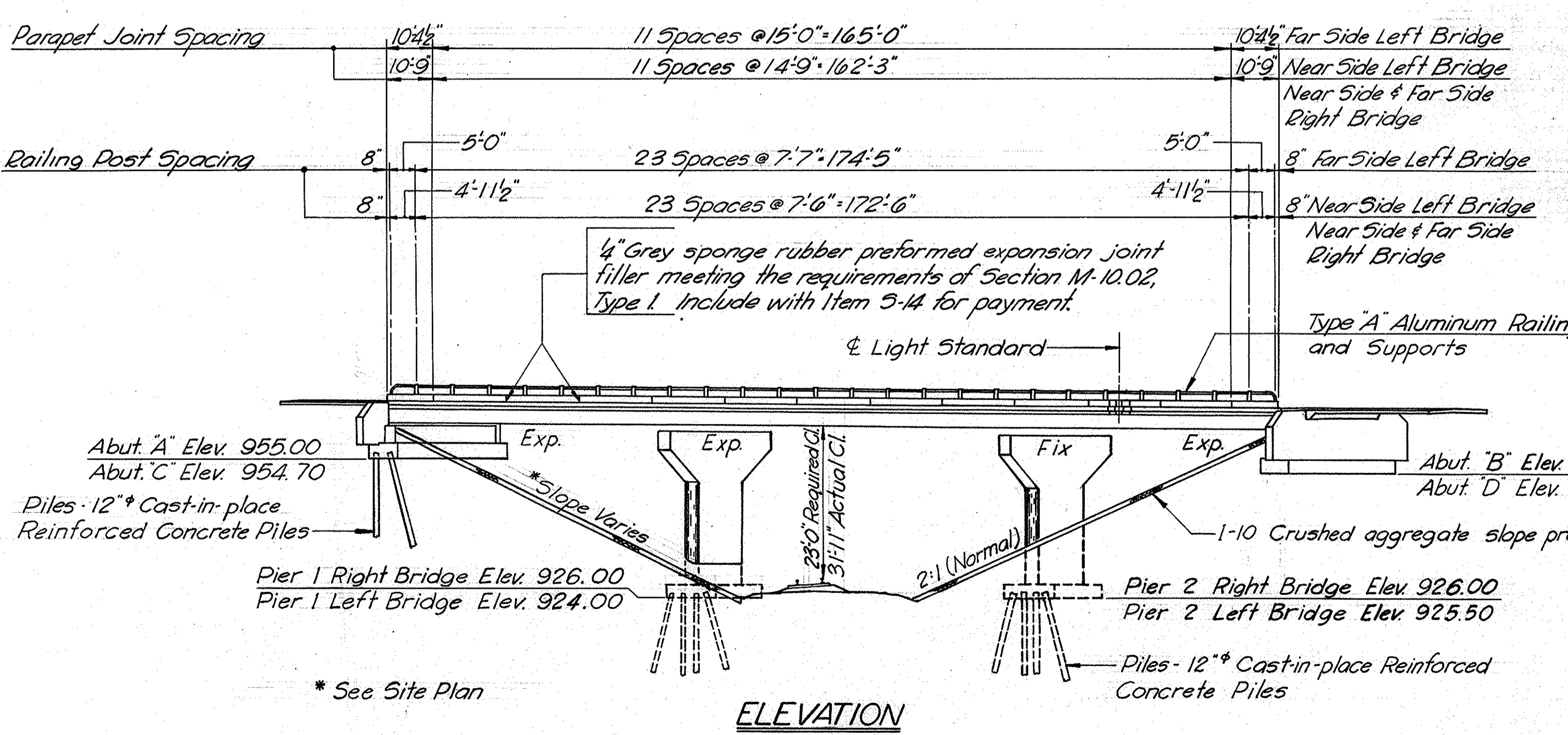
Special care shall be given to the construction of the embankment between Stations 591+50 and 594+00 (parallel to abutments). All soils placed in embankment within these limits shall be spread in layers not greater than 6" loose depth and shall be compacted to not less than 102 percent of Laboratory Maximum Dry Weight. Each lift shall be approved before material for the next lift is brought in.

All shale or rock placed as fill in this area shall be spread in layers not greater than 8 inches loose depth. Rock which cannot be incorporated in an 8 inch lift shall either be reduced in size or excluded from this part of the embankment. All shale layers and layers containing a mixture of shale and rock shall be sprinkled as directed by the Engineer prior to or during compaction until the moisture content of the shale is not less than optimum minus 3 percent and not more than optimum. Each shale or rock layer shall be rolled with at least four complete coverages of a fully ballasted tamping roller and at least two complete coverages of fully ballasted 50 Ton. pneumatic tired roller as specified in Supplemental Specification CE-101.04. All other provisions of Section E-108 of the general specifications shall remain in effect.



TOP OF SLAB ELEVATIONS FOR LEFT BRIDGE - Station 591+78.30 - End Normal Crown and begin Superelevation Runout

STATION	Left Gutter El.	Profile Grade El.	Right Gutter El.
591+78.30	965.16	965.66	965.47
591+90	965.02	965.53	965.37
592+00	964.91	965.42	965.29
592+10	964.79	965.31	965.20
592+20	964.68	965.20	965.15
592+30	964.56		



Utility Lines
All expense involved in relocating affected utility lines shall be born by the owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

Extreme care must be used during the construction of these bridges due to the existing Ohio Edison Company power lines. These lines are located approximately in the area of the forward piers and are 65 ± ft above the final location of the pier piling.

REFERENCE shall be made to Supplemental Specification 3307, revised 10-1-64 and Standard Drawing SD-2-64 dated 11-25-64.

DESIGN LOADING: Cf 2000 (57)

CONCRETE CLASS "C": Basic unit stress 1333 ps.i.

CONCRETE CLASS "E": Basic unit stress 1133 ps.i.

STRUCTURAL STEEL: ASTM A36 - basic unit stress 20,000 ps.i. (ASTM A7 and A373 steel not permitted)

REINFORCING STEEL: ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 ps.i. Except spiral reinforcement may be plain, structural Grade with basic unit stress of 18,000 ps.i.

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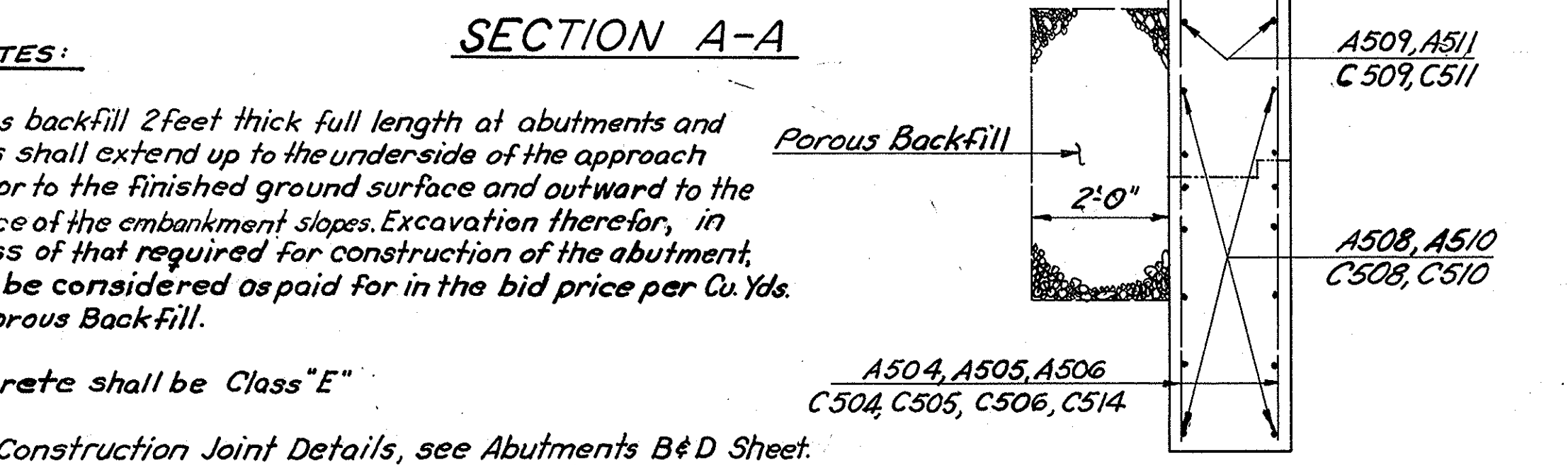
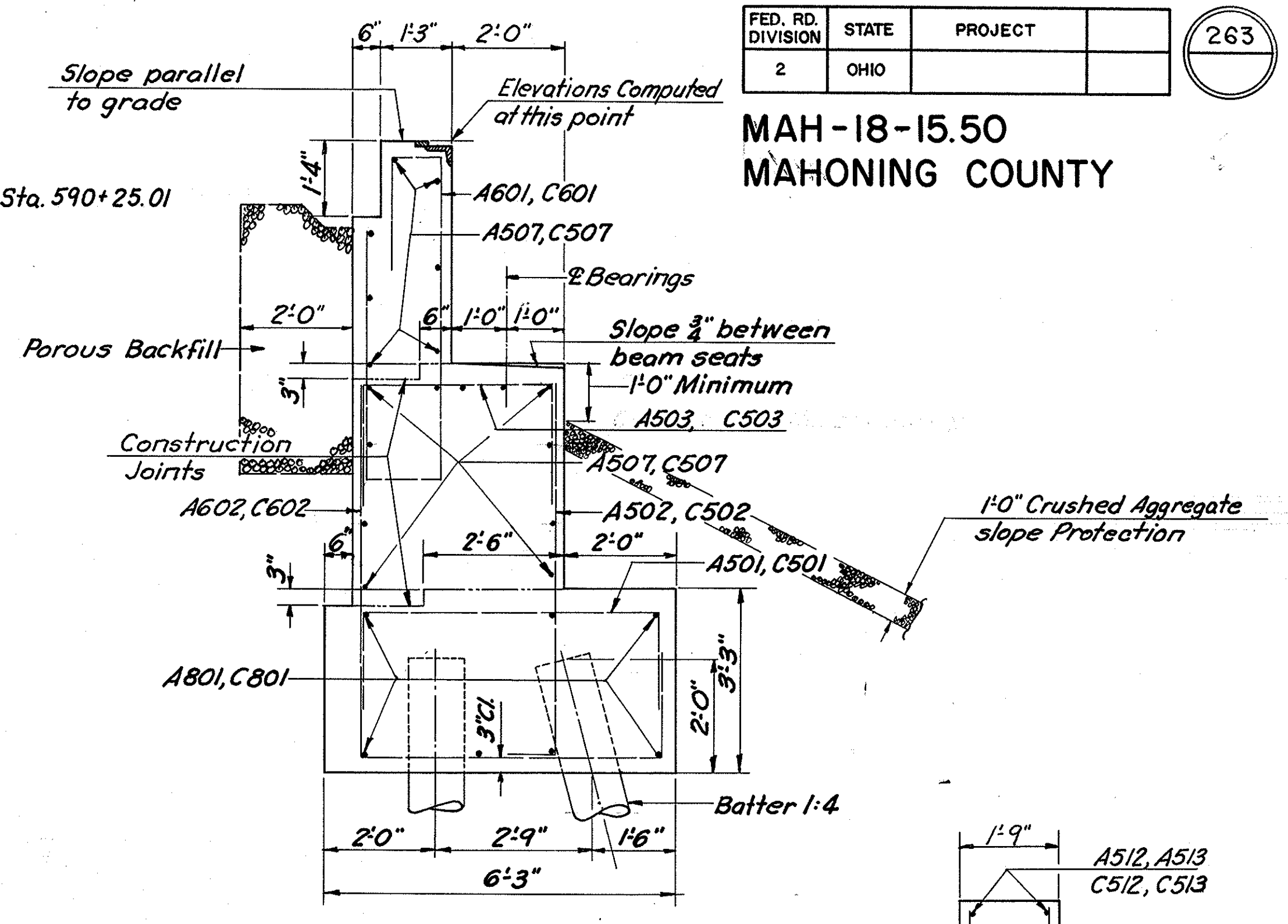
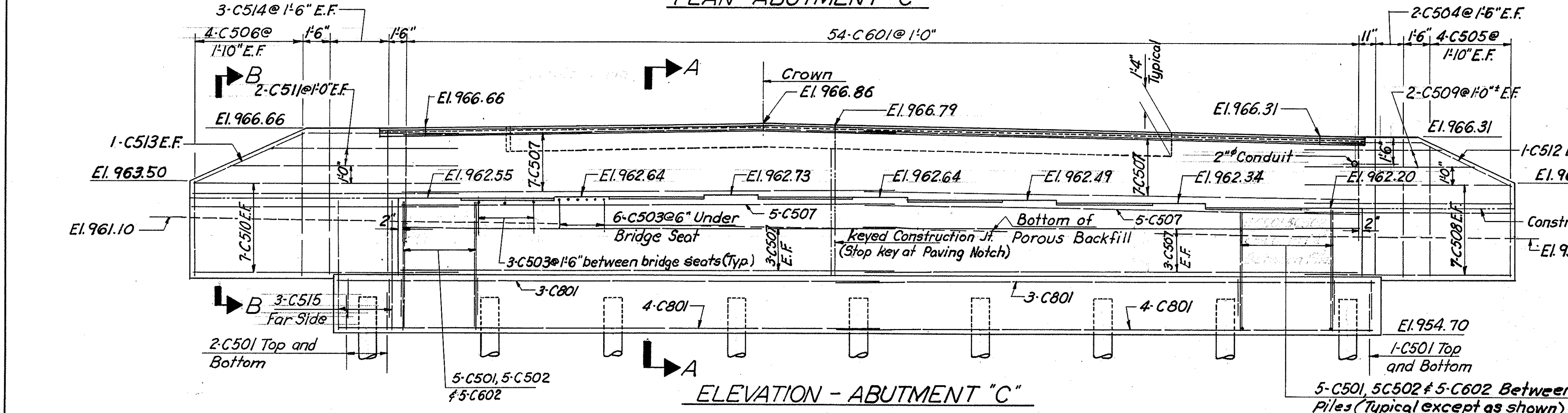
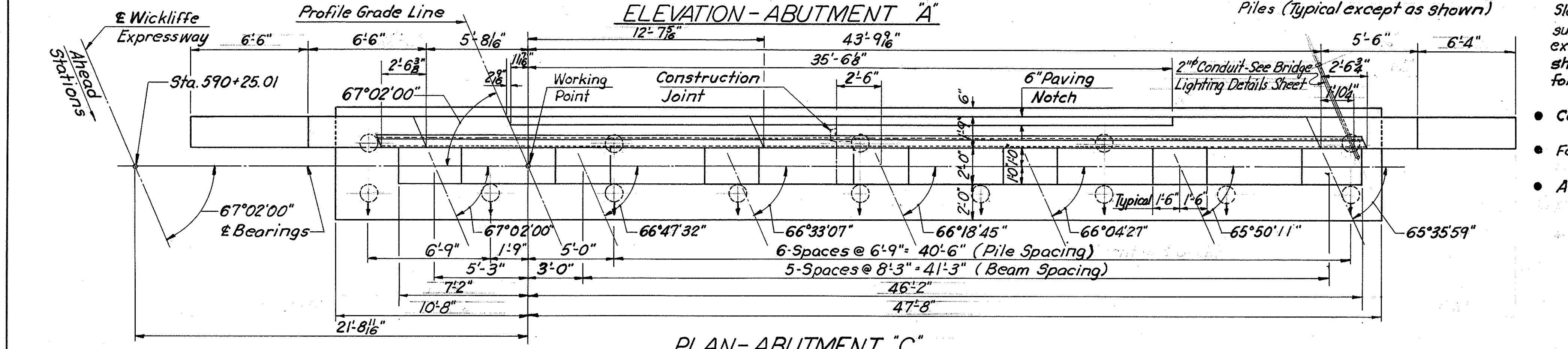
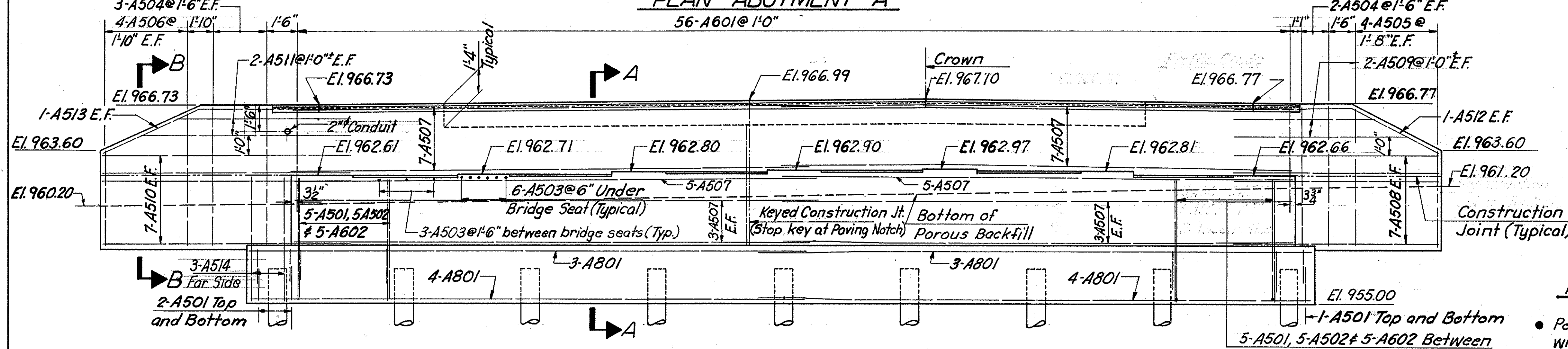
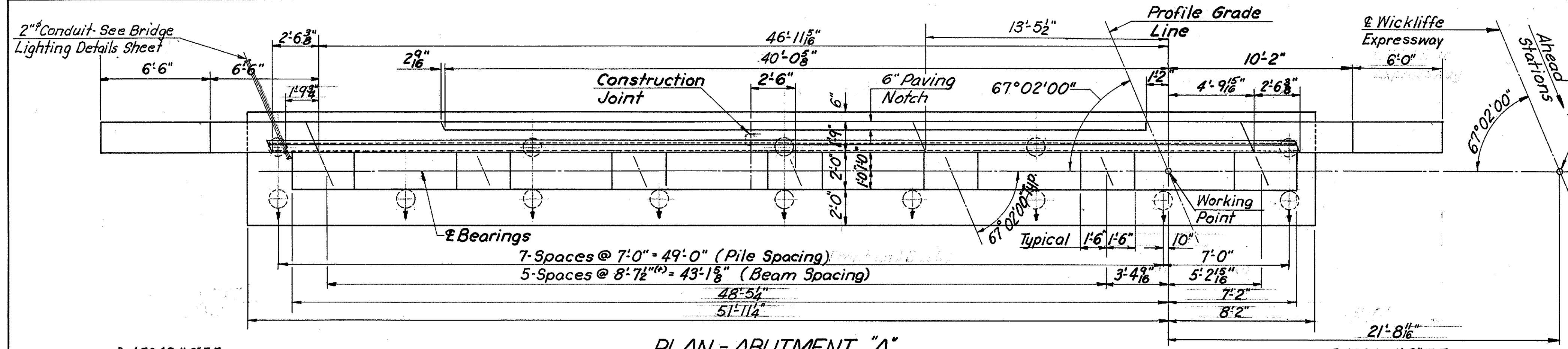
GENERAL PLAN & ELEVATION
BRIDGE NO. MAH-18-1688 L.&R.
OVER ERIE - LACKAWANNA R.R.

MAHONING COUNTY

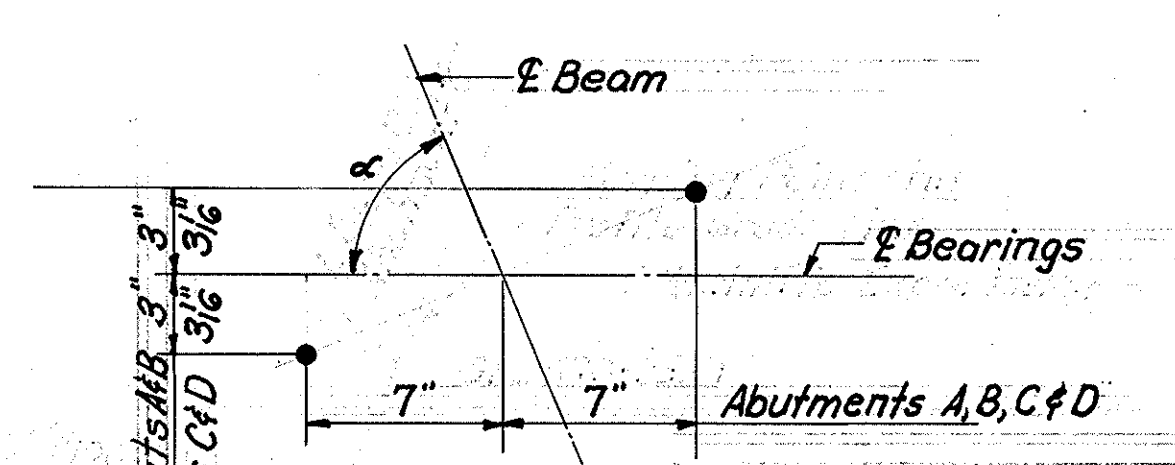
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STA. 590+22.57
STA. 592+09.46

MAH-18-15.50
MAHONING COUNTY



- NOTES:**
- Porous backfill 2 feet thick full length at abutments and wings shall extend up to the underside of the approach Slab or to the finished ground surface and outward to the surface of the embankment slopes. Excavation therefor, in excess of that required for construction of the abutment, shall be considered as paid for in the bid price per Cu. Yds. for Porous Backfill.
 - Concrete shall be Class "E"
 - For Construction Joint Details, see Abutments B&D Sheet.
 - All piles to be 12" Cast-in-Place Reinforced Concrete Piles.



NOTE: For angle α , refer to "Abutment Plan."

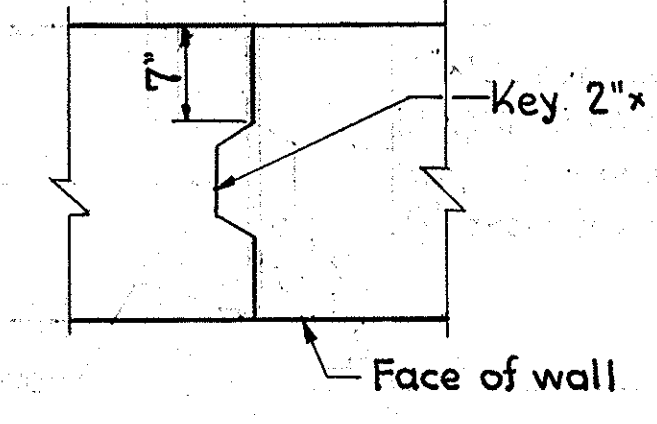
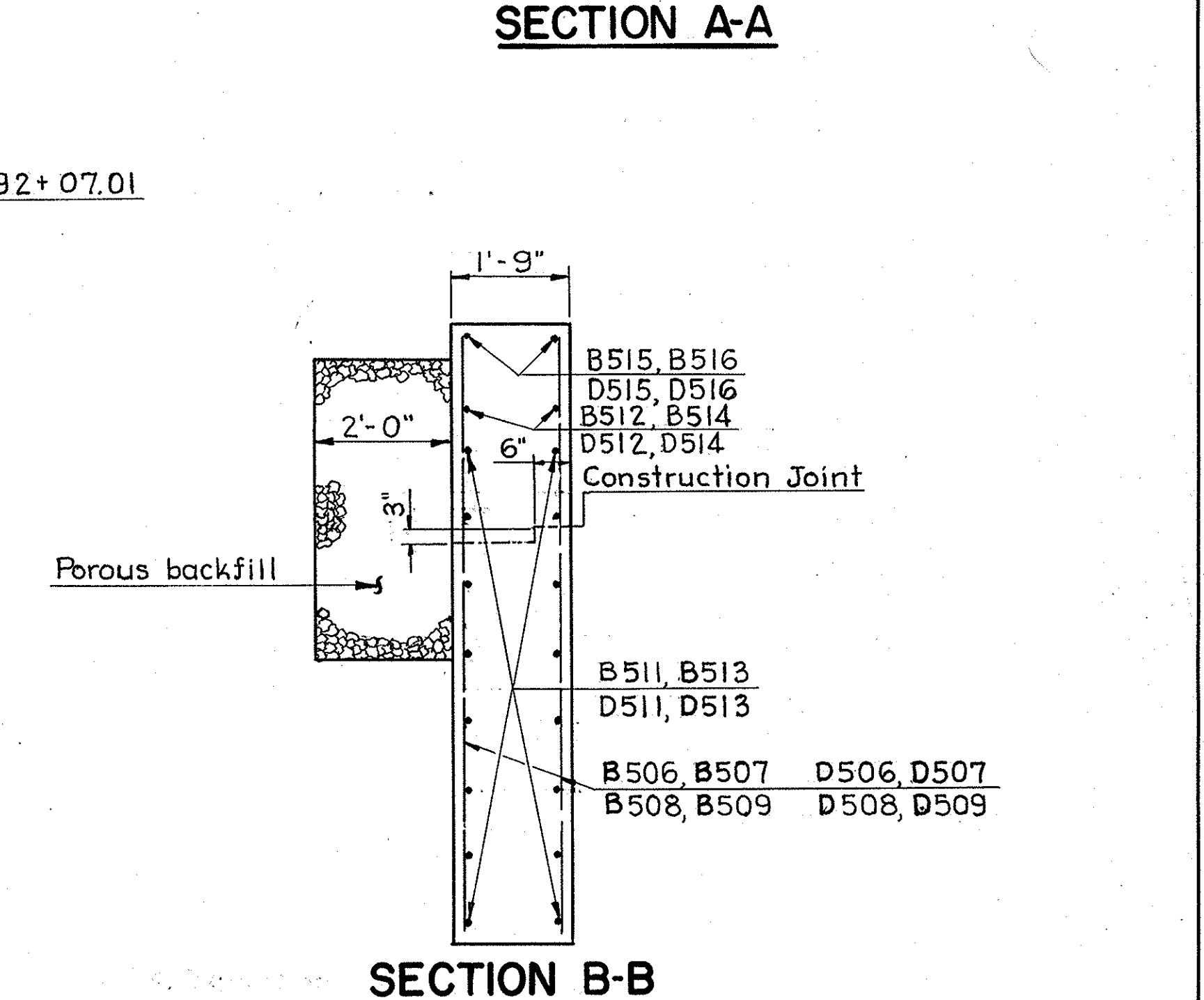
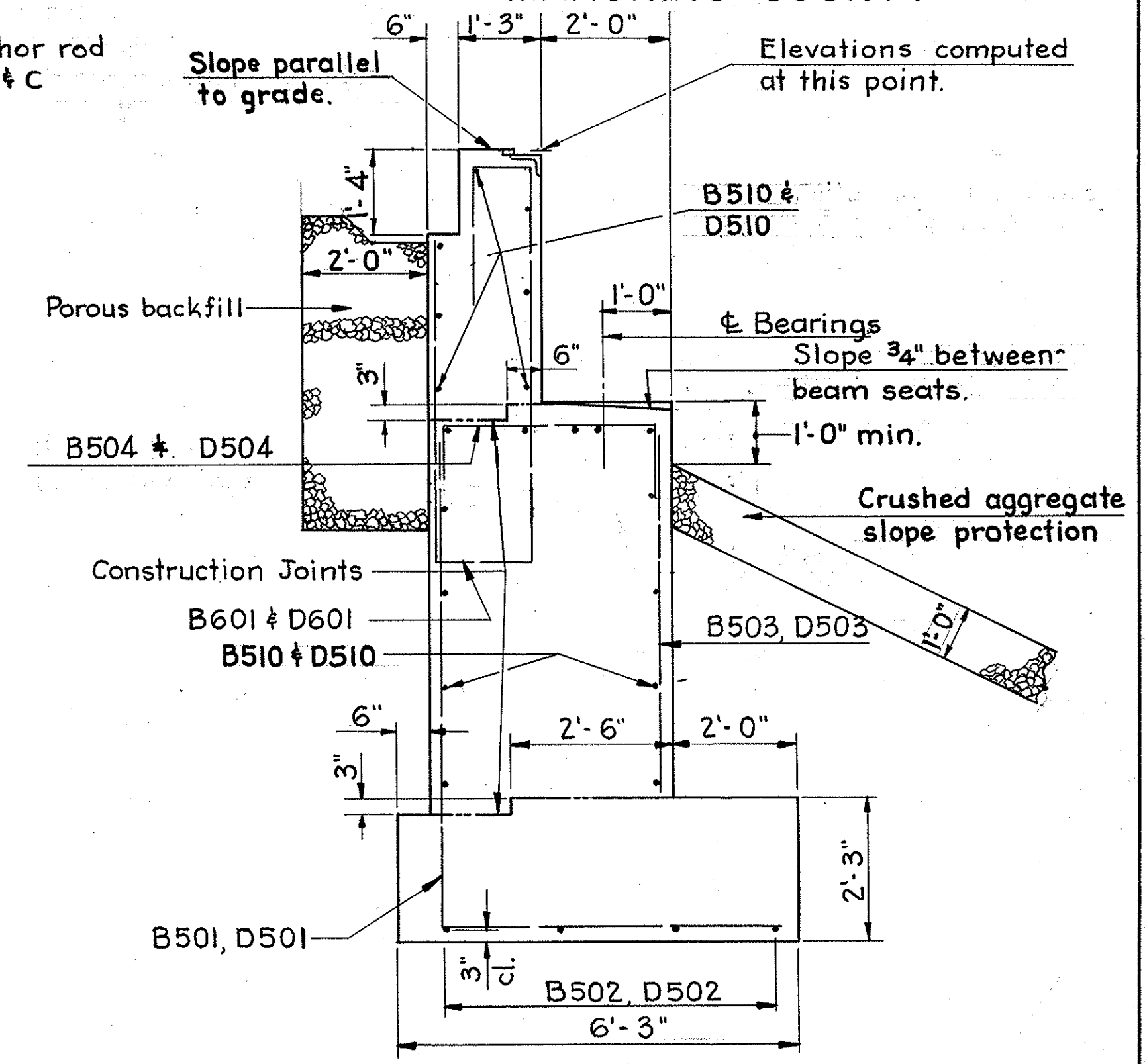
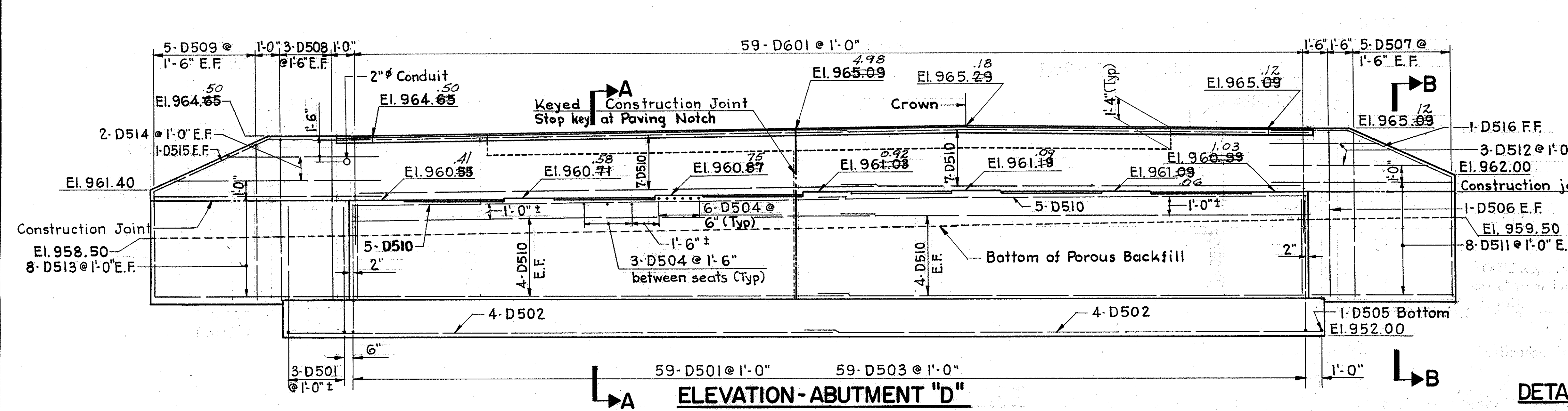
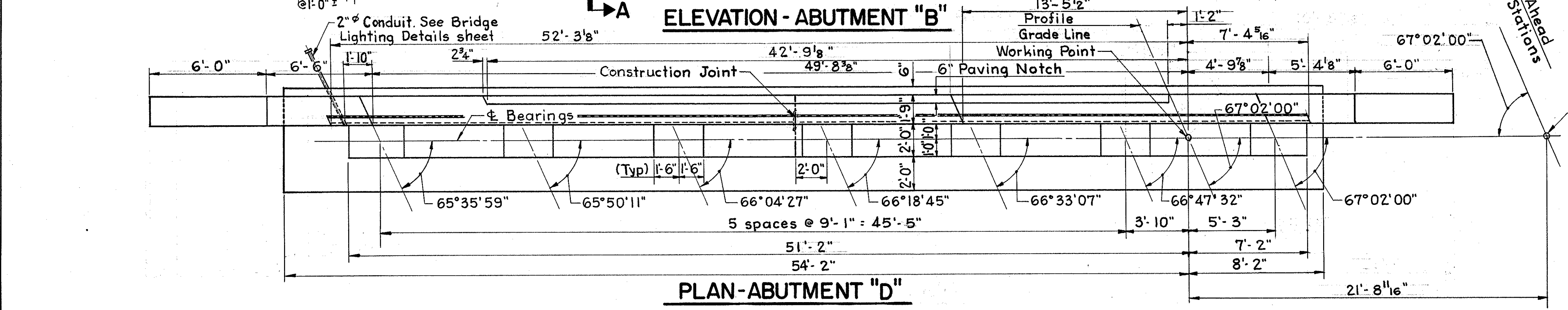
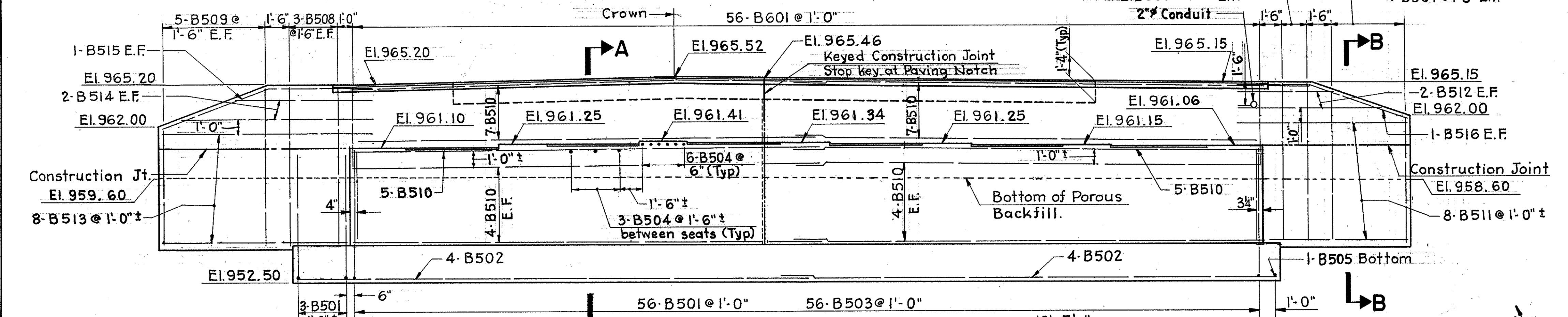
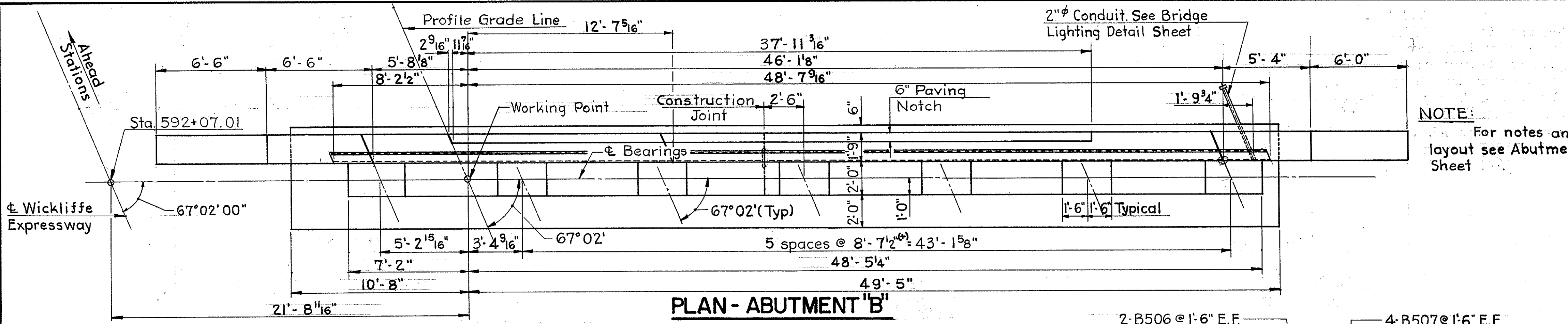
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

ABUTMENTS A & C
BRIDGE NO. MAH-18-1688 L.&R.
OVER ERIE - LACKAWANNA R.R.

MAHONING COUNTY
STA. 590+22.57
STA. 592+09.46

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
DWN	DWN	RJ	EVR	DFR	8-16-65	

**MAH-18-15.50
MAHONING COUNTY**



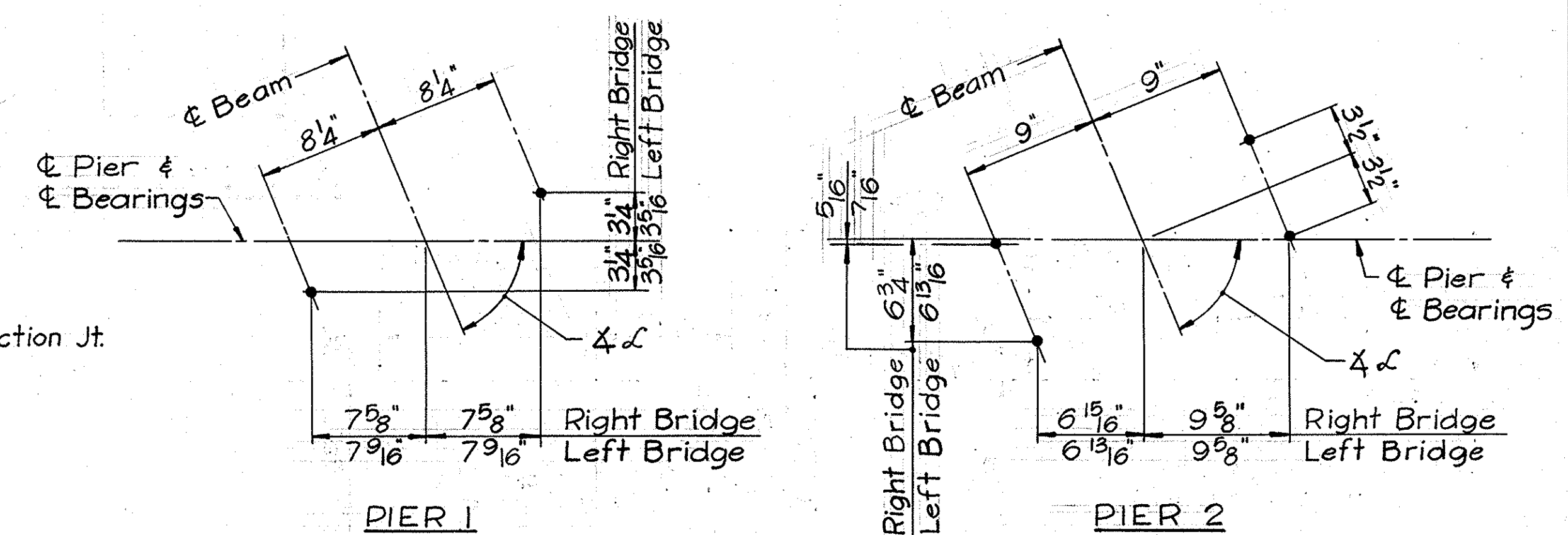
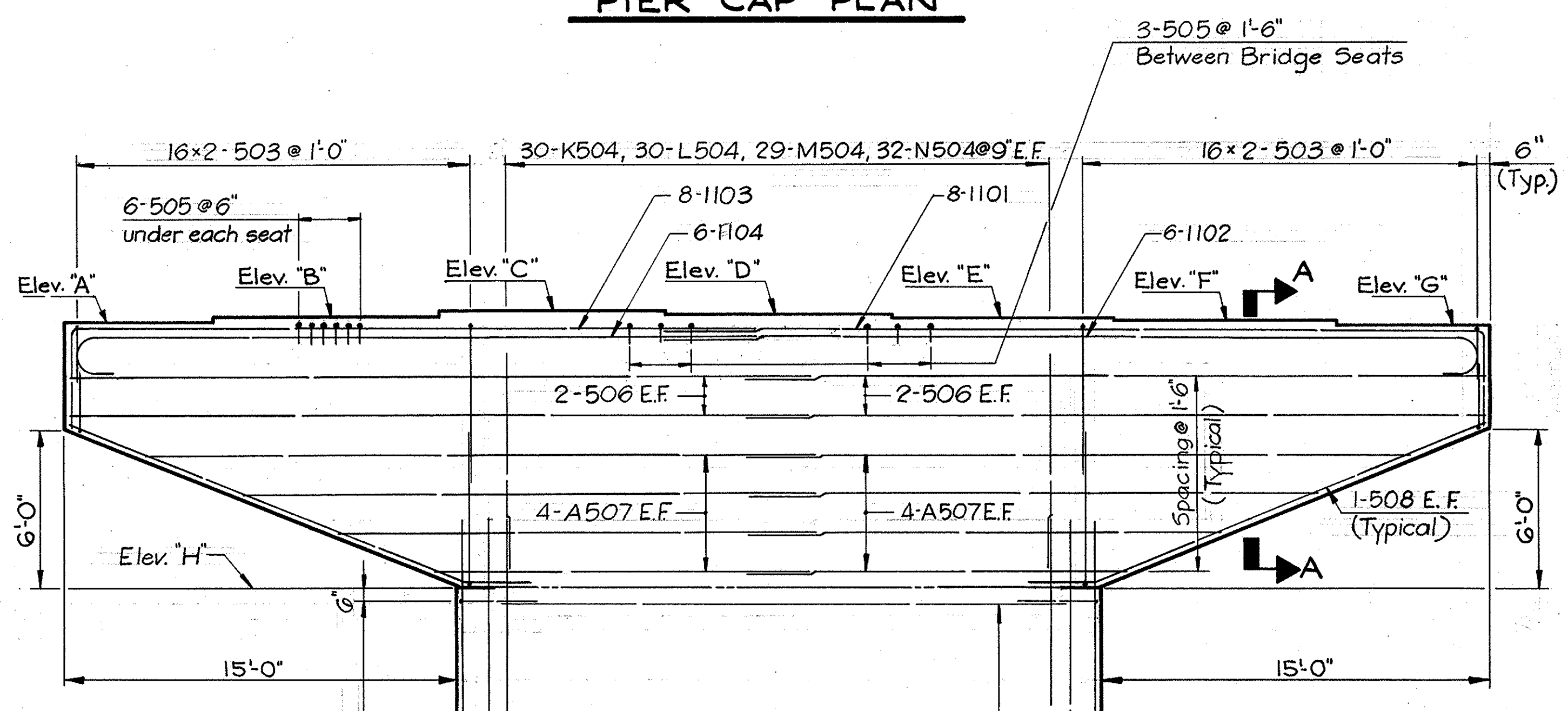
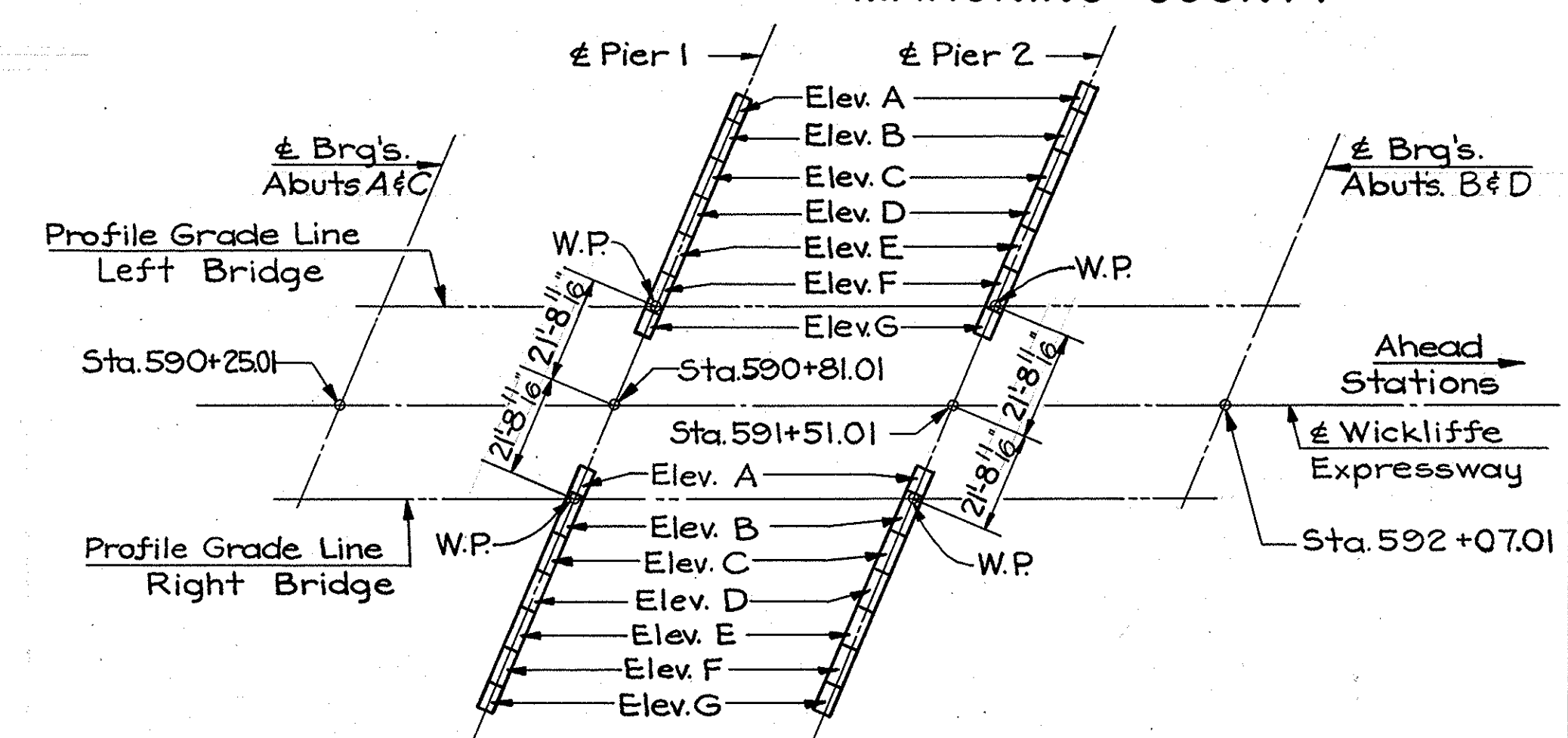
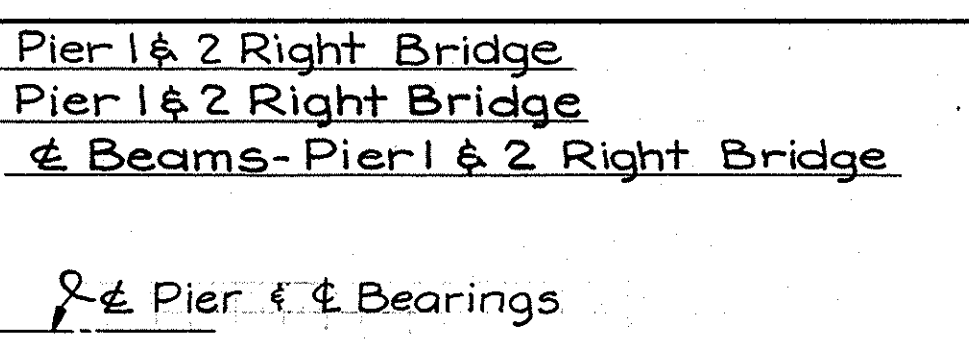
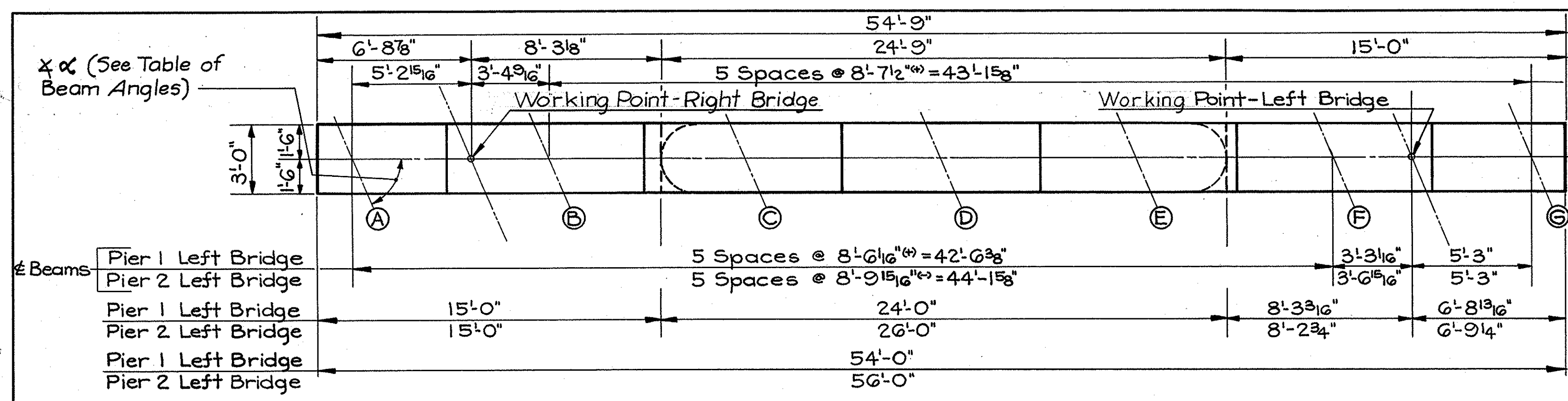
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

ABUTMENTS B & D
BRIDGE NO. MAH-18-1688 L & R.
OVER ERIE - LACKAWANNA R.R.

MAHONING COUNTY STA. 590+22.57
STA. 592+09.46

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
DMN	DMN	R.T.R.	V.K.	MBJ	8-16-63	2-1-66

**MAH-18-15.50
MAHONING COUNTY**



NOTE:
BARS ARE DESIGNATED THUS:
K - Pier 1 Right
L - Pier 2 Right
M - Pier 1 Left
N - Pier 2 Left
All bars without prefix are for all piers.

LEGEND:
E.F. = Each Face Same as Right Side
⊙ = Battered Piles

NOTES:
• Super-Structure Ground-A No. 1/8 AWG 7 strand soft annealed bare copper cable shall be encased in the outside of pier No. 2 left and right bridge. Connect (by Exothermic weld process) lower end of cable to metal shell of piles. Extend cable (in one continuous length) through top of pier with lead of sufficient length to exothermic weld upper end of cable to outside beam of super-structure.
• Payment for electrical grounds is included in the lump sum bid for 5-25 "Electrical Lighting System Complete."

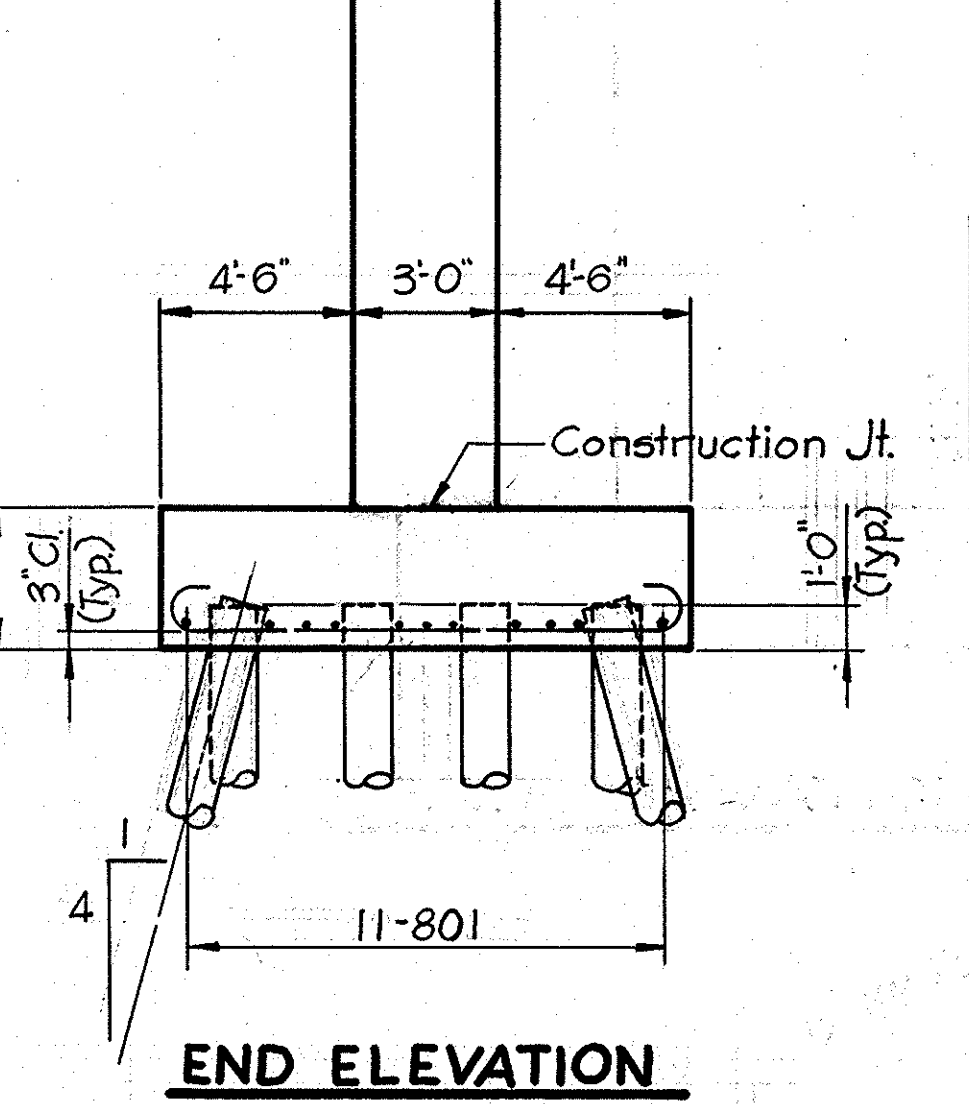
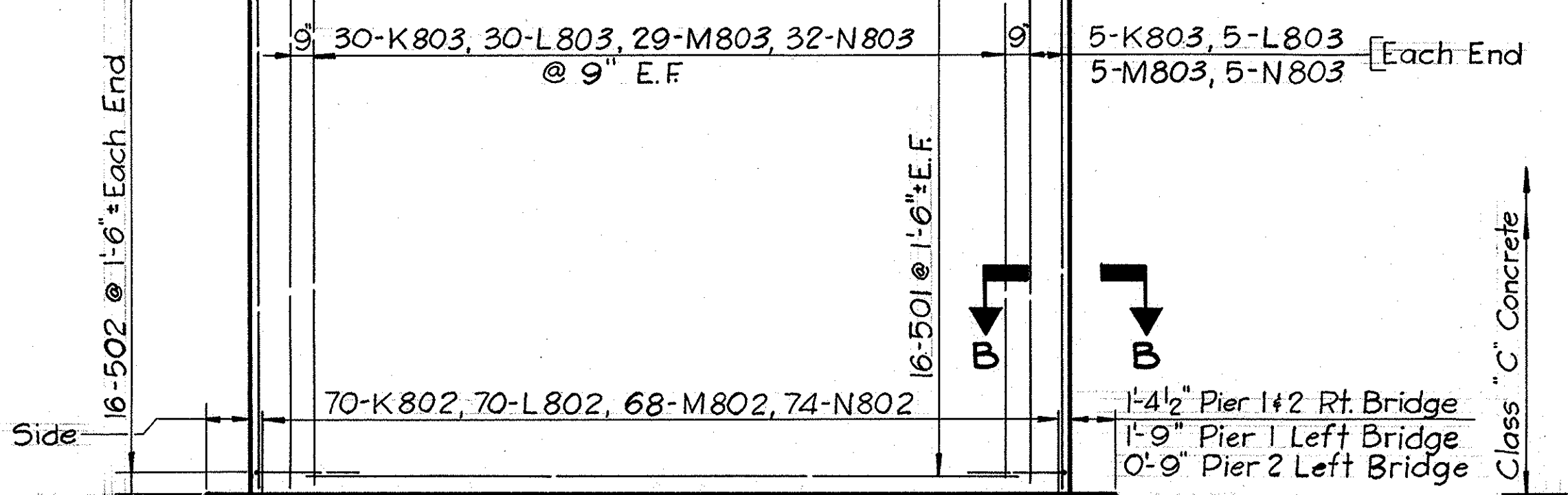


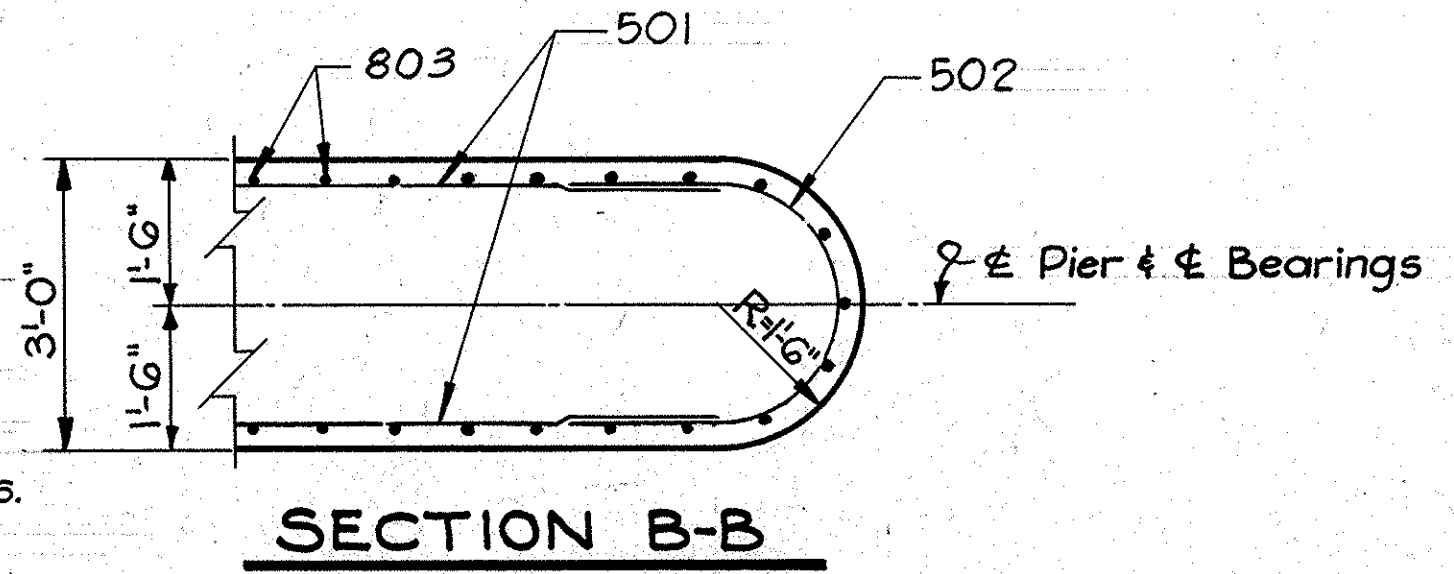
TABLE OF BEAM ANGLES

Location	Dimension	A	B	C	D	E	F	G
Pier 1 & 2 Left	4 ∟	65°35'53"	65°50'11"	66°04'27"	66°18'45"	66°33'07"	66°47'32"	67°02'00"
Pier 1 & 2 Right	4 ∟	67°02'00"	67°02'00"	67°02'00"	67°02'00"	67°02'00"	67°02'00"	67°02'00"

TABLE OF ELEVATIONS

Location	A	B	C	D	E	F	G	H	J
Pier 1 Right	962.14	962.29	962.44	962.38	962.28	962.19	962.09	952.09	926.00
Pier 2 Right	961.25	961.40	961.55	961.49	961.39	961.30	961.20	951.20	926.00
Pier 1 Left	961.65	961.80	961.95	962.10	962.22	962.12	962.03	951.65	924.00
Pier 2 Left	960.72	960.88	961.04	961.19	961.33	961.23	961.14	950.72	925.50

NOTES:
• Special care shall be taken in placing reinforcing steel in the bridge seat so that it will not interfere with the drilling of anchor rod holes.
• Batter piles 1 in 4 as shown on the FOOTING PLAN.
• All piles to be 12" Cast-in-place Reinforced Concrete Piles.



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ROCHESTER, PENNSYLVANIA

PIERS
BRIDGE NO. MAH-18-1688 L. & R.
OVER ERIE - LACKAWANNA R.R.

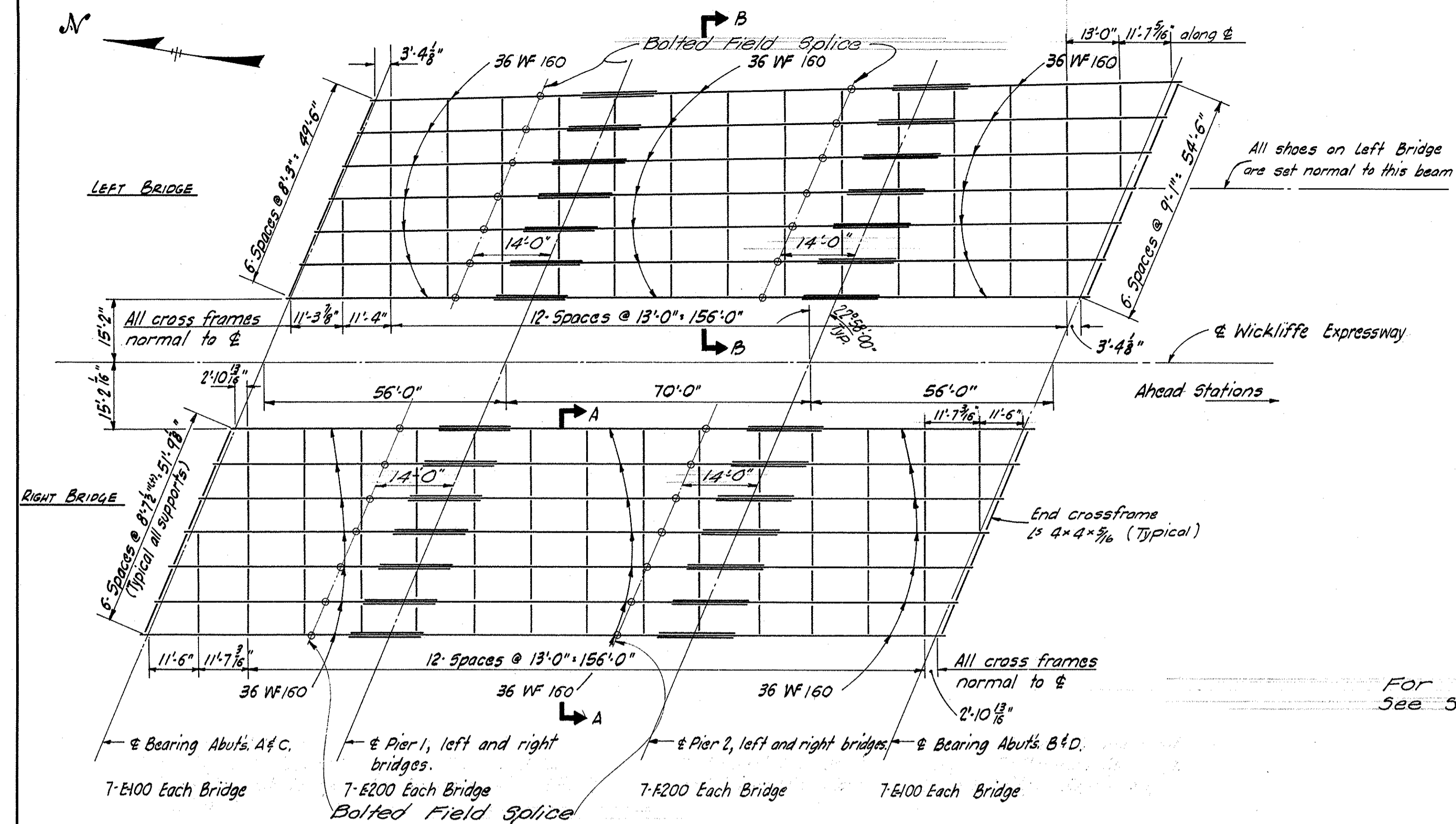
MAHONING COUNTY
STA. 590 + 22.57
STA. 592 + 09.46

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
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MAH-18-15.50
MAHONING COUNTY

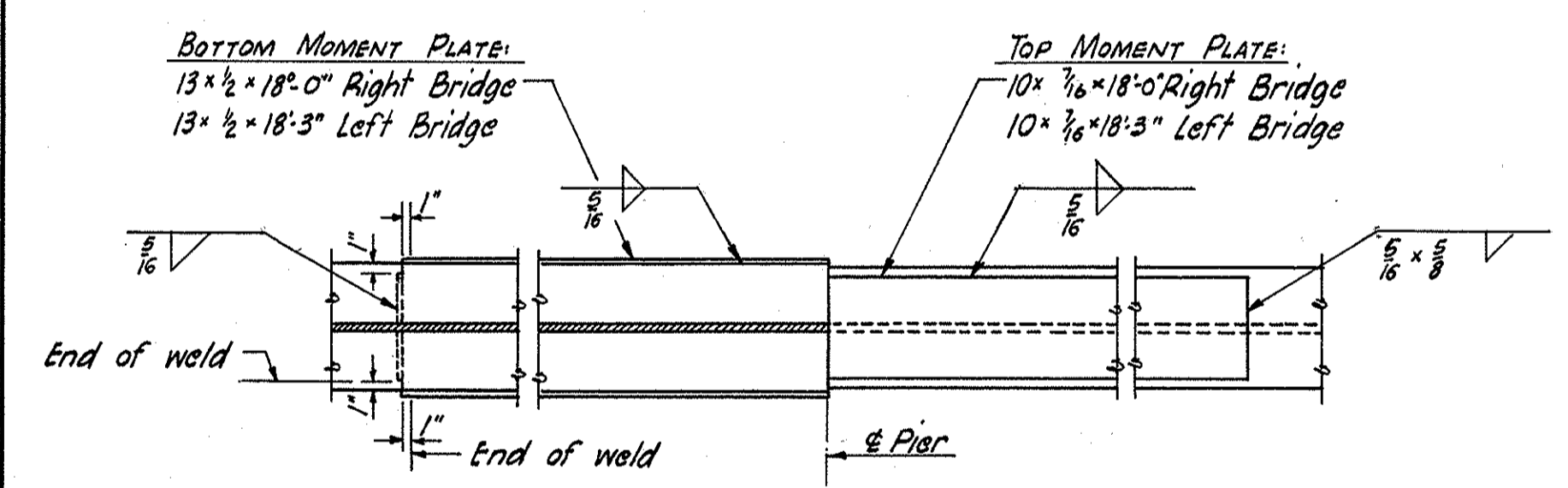
DEFLECTION & CAMBER			
LOCATION	SPAN 1	SPAN 2	SPAN 3
Deflection due to weight of steel	1/16"	1/16"	1/16"
Deflection due to remaining Dead Load	1/4"	3/8"	1/4"
Convexity required for vertical curve	0"	0"	0"
Sum of deflections and convexity	5/16"	7/16"	5/16"
Required Camber	0"	0"	0"

Place any natural camber up.



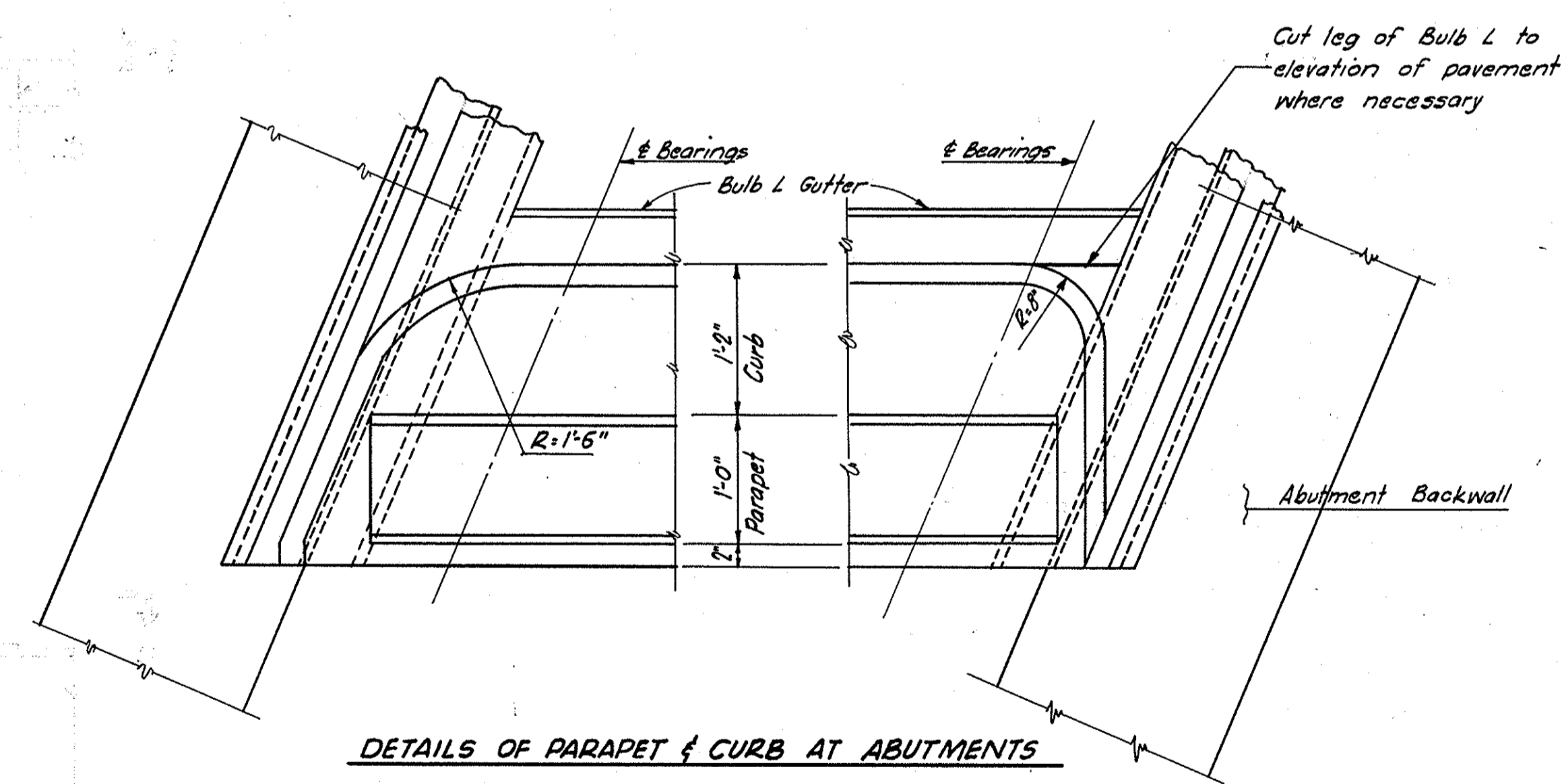
STEEL FRAMING PLAN

For BOLTED FIELD SPLICES DETAILS See Standard Drawing 3D-2-64

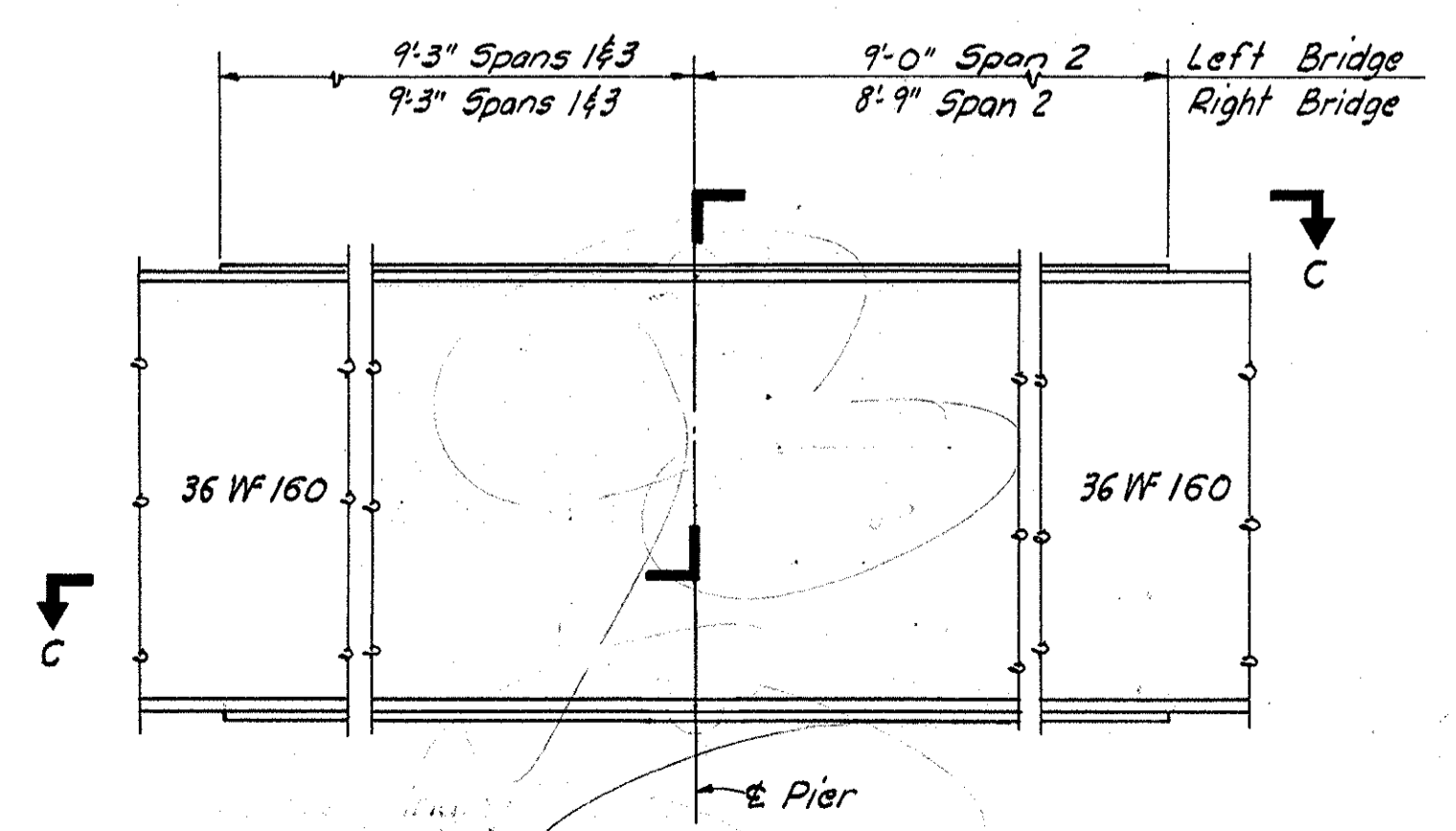


SECTION C-C

NOTE: See other Superstructure Detail Sheet for Sections A-A & B-B



DETAILS OF PARAPET & CURB AT ABUTMENTS



MOMENT PLATE DETAIL

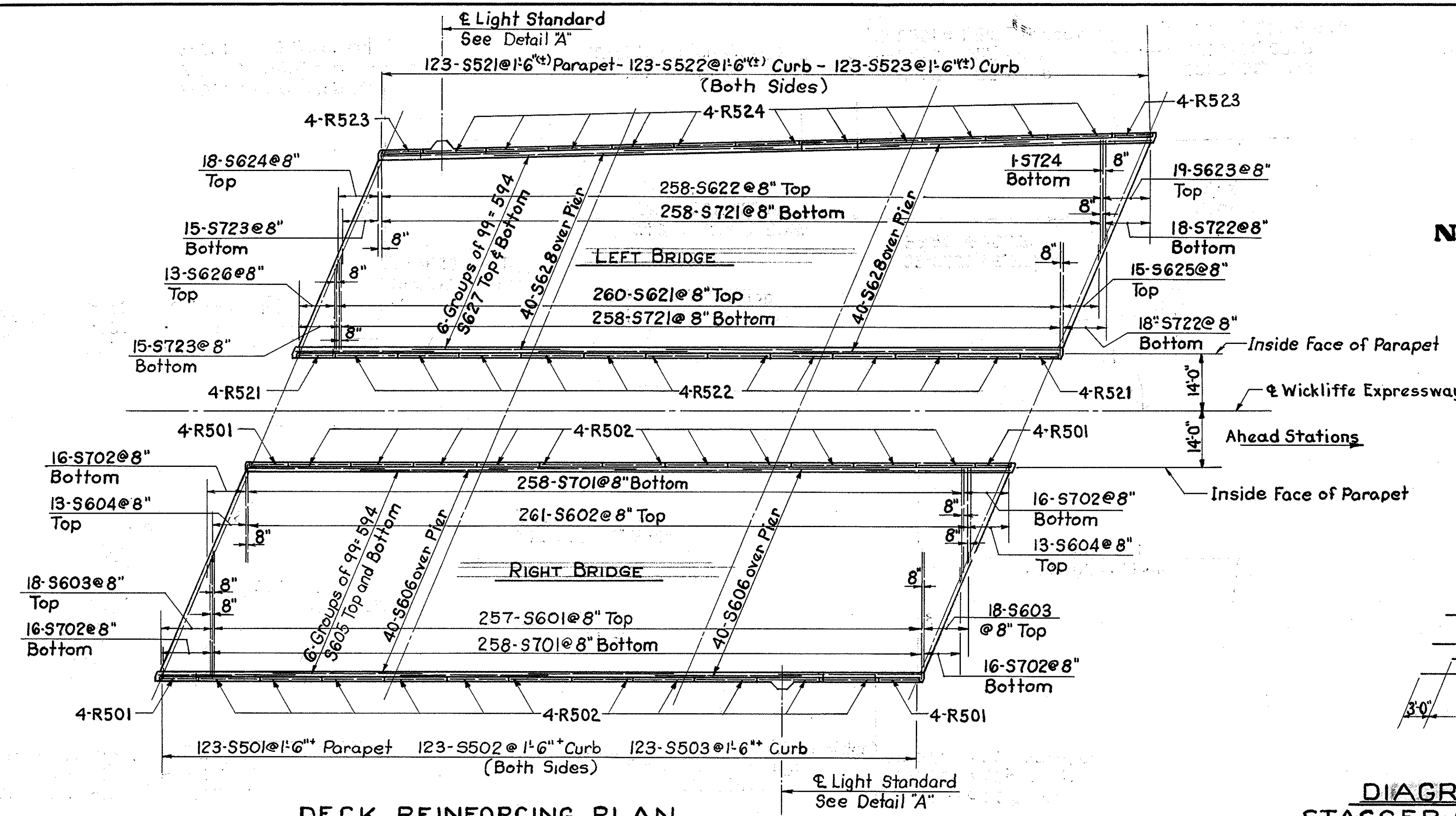
MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
SUPERSTRUCTURE DETAILS					
BRIDGE NO. MAH-18-1688 L & R.					
OVER ERIE - LACKAWANNA R.R.					
MAHONING COUNTY					
STA 590+22.57					
STA 592+09.46					
Designed	Drawn	Traced	Checked	Reviewed	Date
D.M.	J.M.	M.M.	V.K.	J.P.K.	8-16-63

**MAH-18-15.50
MAHONING COUNTY**

NOTES:

- Refer to Standard Drawing CSB-2-56, Sheets 2 of 6 for details of End Dams, End Crossframes, 2" Pipe Drains, Scuppers and to Standard Drawing AR-1-57 for details of Type "A" Aluminum Railing and Concrete Parapet and to Standard Drawing FSB-1-62 for details of fixed and sliding bearings.
- CONCRETE shall be class "C" Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
- See General Plan and Elevation for location of scuppers, 2" pipe drains, parapet joints and railing posts.
- CONCRETE DECK PLACING: In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress upgrade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
- DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width.
- MACHINE FINISH: The concrete bridge deck shall be finished by the use of a finishing machine.

CONTINUOUS ALUMINUM RAILING AND CONCRETE PARAPET For the purpose of checking the fit of the railing and parapet on the bridge beams, the contractor shall be responsible for providing the correct dimensions and positions. All beams shall be marked with red paint and the railing and parapet shall be installed in accordance with the above dimensions.



DECK REINFORCING PLAN

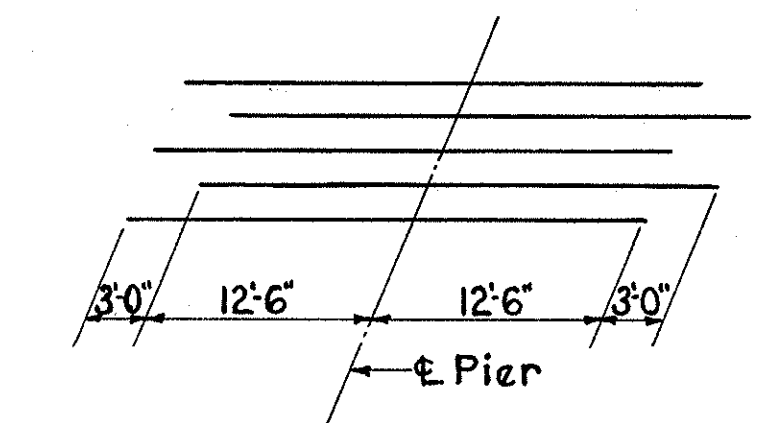
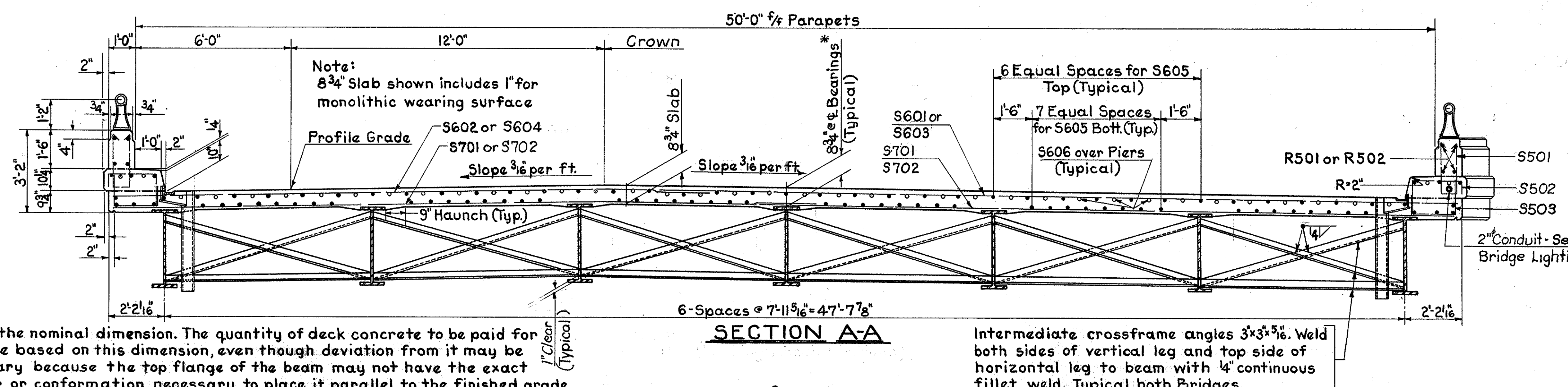
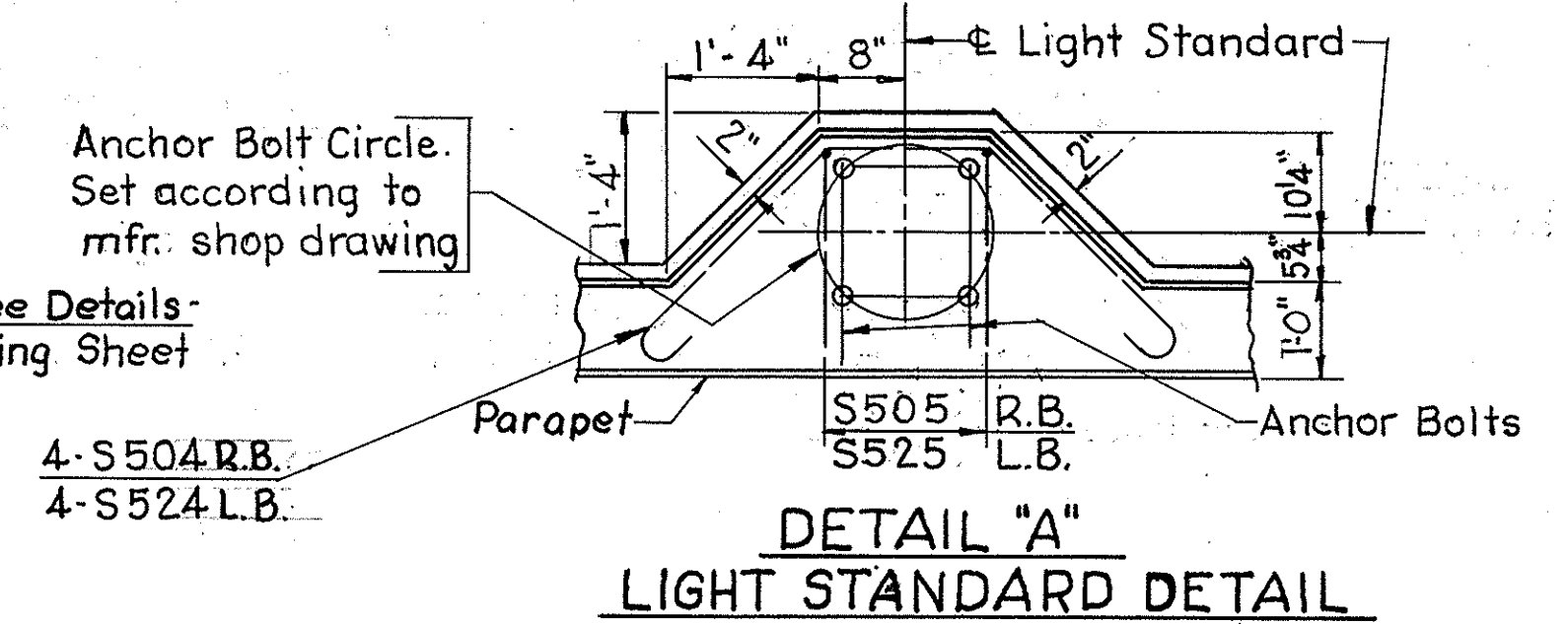


DIAGRAM SHOWING STAGGER OF S606 & S628 BARS



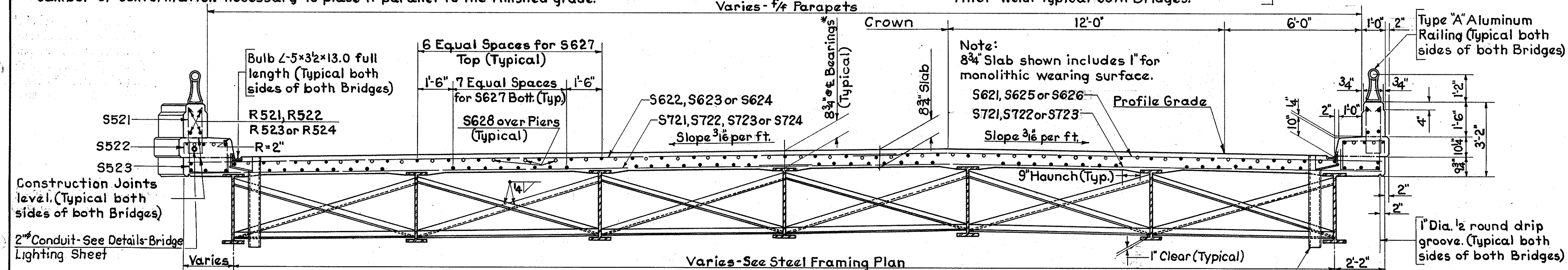
SECTION AA



DETAIL "A" LIGHT STANDARD DETAIL

*This is the nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation necessary to place it parallel to the finished grade.

Intermediate crossframe angles 3"x3"x5/16". Weld both sides of vertical leg and top side of horizontal leg to beam with 4" continuous fillet weld. Typical both Bridges.



SECTION B-B

Construction Joints level. (Typical both sides of both Bridges)

Standard Scupper (Typical both sides of both Bridges)

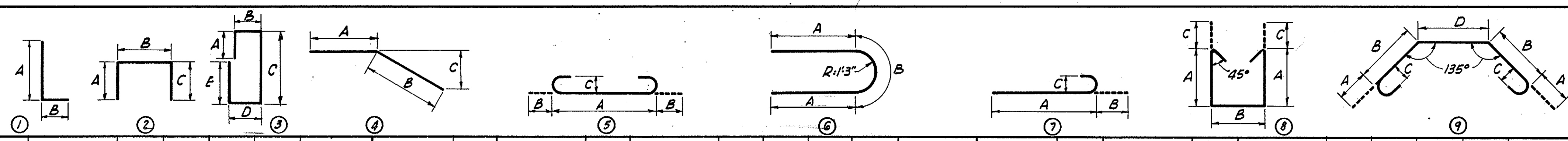
(Deck Slopes Typical Back Of Station 591 + 78.30)

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS
BRIDGE NO. MAH-18-1688 L. & R.
OVER ERIE - LACKAWANNA R.R.

MAHONING COUNTY STA 590+22.57
STA. 592+09.46

Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
DWN	DWN	CPM	JK	MBK	5-16-63	2-15-66



Main table containing material specifications, dimensions, and weights for bridge components. Includes sections for ABUTMENT A, ABUTMENT B, ABUTMENT C, ABUTMENT D, PIER 1 (LEFT), PIER 1 (RIGHT), PIER 2 (LEFT), PIER 2 (RIGHT), SUPERSTRUCTURE - RIGHT BRIDGE, and SUPERSTRUCTURE - LEFT BRIDGE.

ESTIMATED QUANTITIES

Table of estimated quantities for bridge construction, listing items like excavation, concrete, steel, and lighting systems with their respective units and quantities for right and left structures.

BAR SIZE

Bar size is indicated in the bar mark. The first digit where three digits are used, and the first two digits where four are used, indicate the bar size number. For example, A601 is a number 6 size bar.

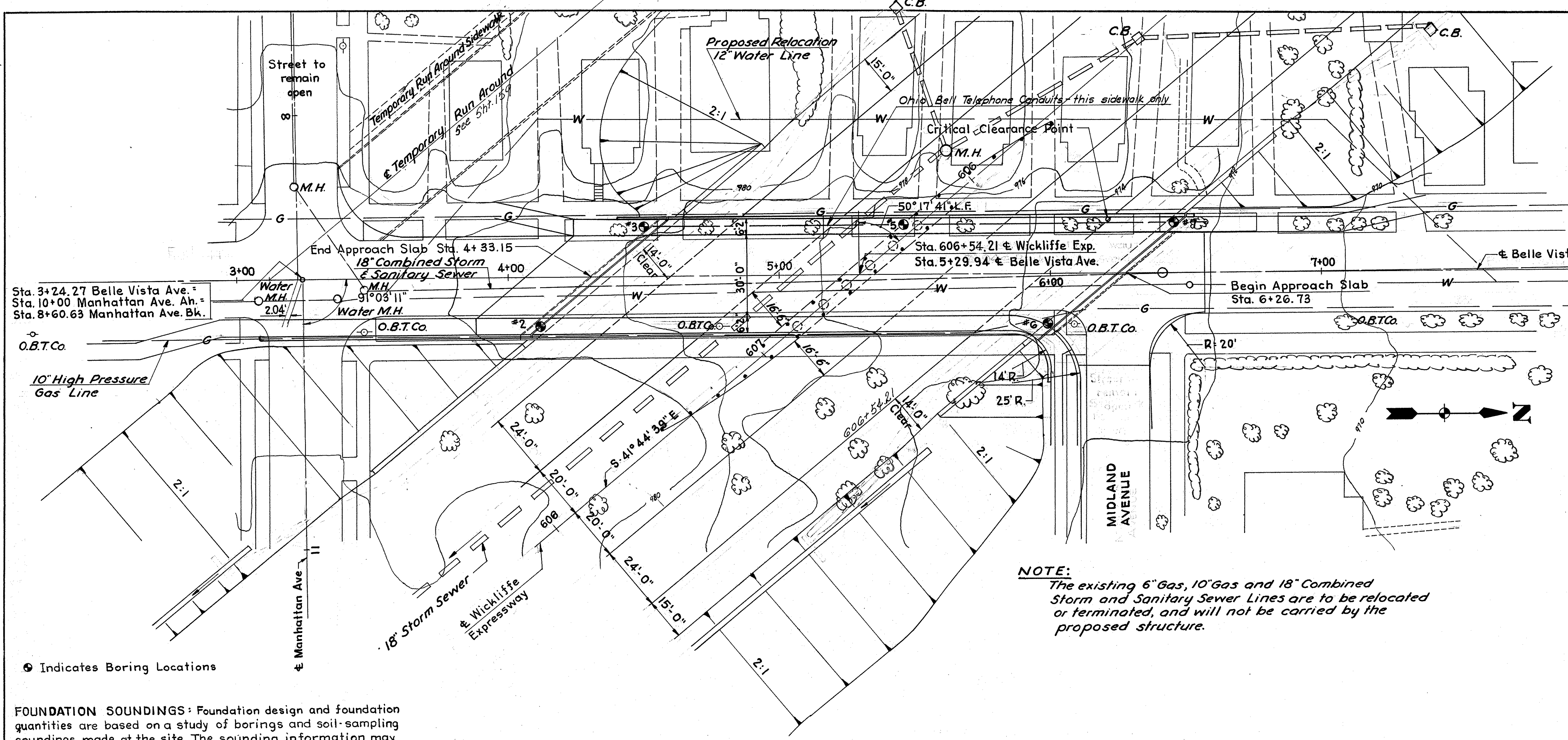
REPLACEMENT BARS

Table showing replacement bar specifications, including mark, total, length, and type for various bar marks like RE1101, RE901, etc.

* See General Summary - Lighting Sheet
** Payment will be made for only one First Test Pile. It may be driven for either the Right or Left Bridge.

Project information block including Michael Baker Jr., Consulting Engineers, Reinforcing Bar Schedule & Estimated Quantities, Bridge No. MAH-18-1688 L & R. Over Erie - Lackawanna R.R., and design details.

MAHONING COUNTY
MAH-18-15.50



6" Low Pressure Gas Line

12" C.I. Water Line

B.M. 107: Top stem of hydrant North side of Manhattan Avenue 32' E. on Manhattan Avenue from Belle Vista Avenue Elev. 984.68.

B.M. 109: Top stem of hydrant N.W. corner Smithfield and Belle Vista Avenue 50' S.W. on Smithfield from Belle Vista Avenue.

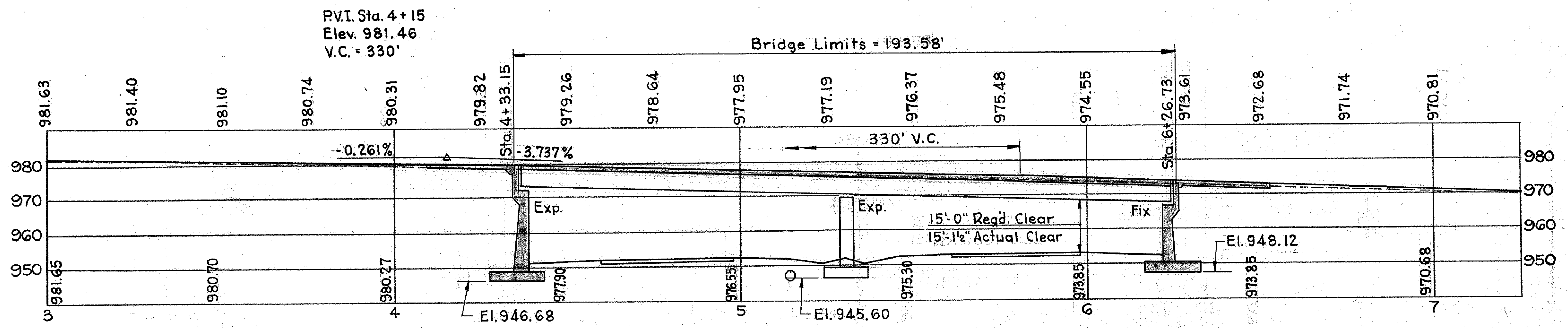
NOTE:
The existing 6" Gas, 10" Gas and 18" Combined Storm and Sanitary Sewer Lines are to be relocated or terminated, and will not be carried by the proposed structure.

● Indicates Boring Locations

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil-sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

A.D.T. (1975) 23,280 (16% Type B & C vehicles)

PROPOSED STRUCTURE
TYPE: Continuous Steel Girder with reinforced concrete deck and substructure
SPANS: 92'-10 1/2", 92'-10 1/2"
ROADWAY: 30' 7/8" 6'-2" Sidewalks
LOAD FREQUENCY: CF 2000 (57)
SKREW: 50° 17' 41" L.F.
WEARING SURFACE: 1" Monolithic Conc.
APPROACH SLAB: Forward-Special Design (25' long) Rear-AS-1-54 (25' long)
ALIGNMENT: Tangent



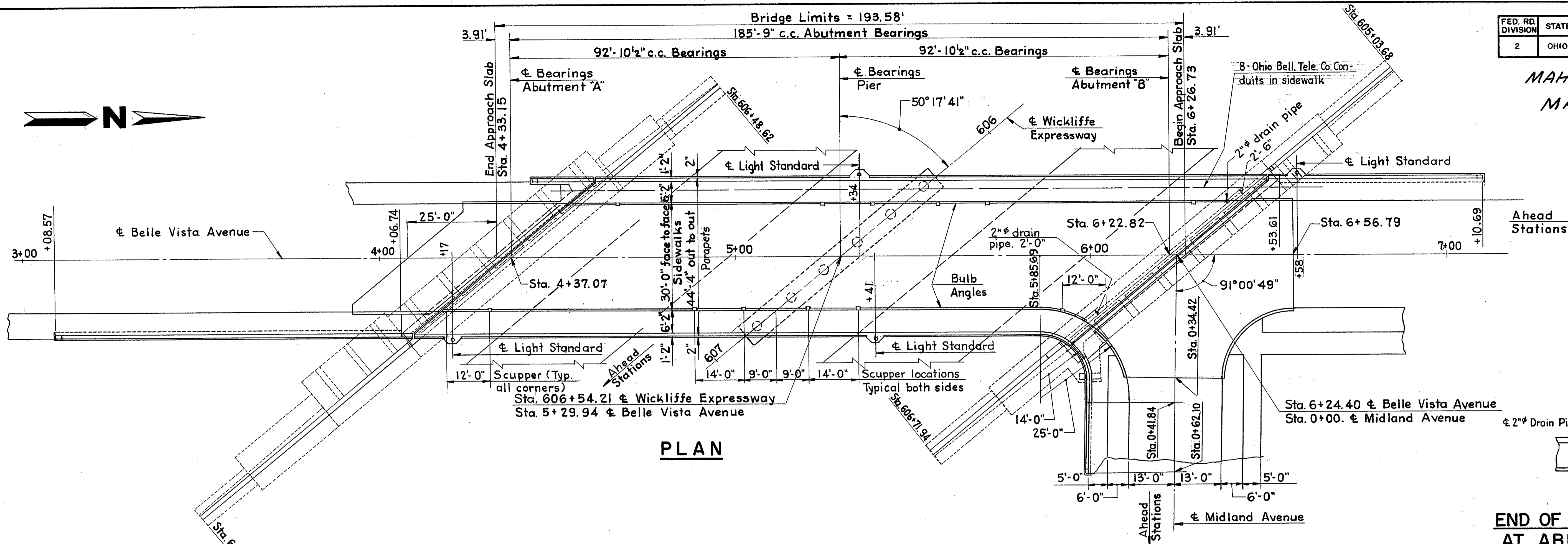
PROFILE ALONG & BELLE VISTA AVENUE

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SITE PLAN
BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

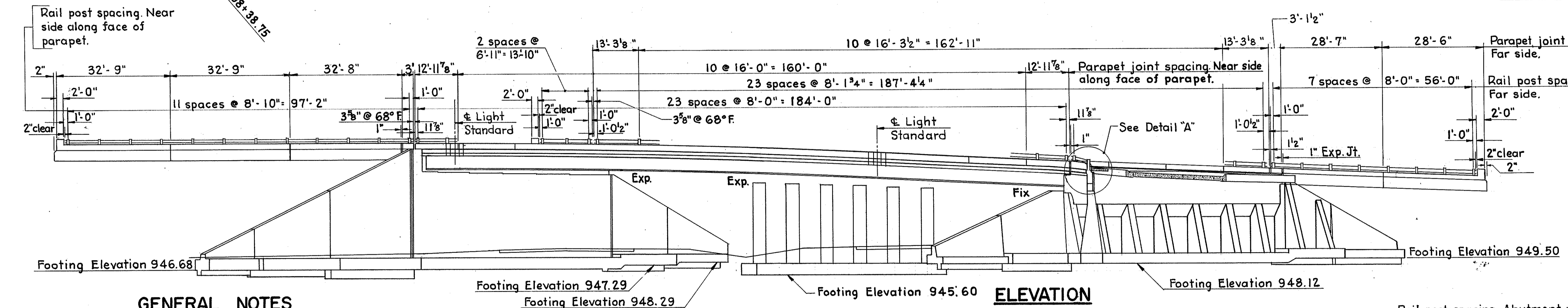
MAHONING COUNTY				STA. 606+54.21			
PRESENT TOPOGRAPHY		DESIGNED		PROPOSED WORK		DRAWN	
SURVEYED	DRAWN	DESIGNED	DRAWN	CHECKED	REVIEWED		
Aurilio	mm	FK	J.H.	E.F.R.	FK	10-16-63	

MAHONING COUNTY
MAH-18-15.50



PLAN

END OF BULB ANGLES AT ABUTMENT B



ELEVATION

GENERAL NOTES

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.

REFERENCE shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59; AR-1-57 revised 4-2-62; FSB-1-62 revised 1-15-63; AS-1-54 revised 7-5-62; Supplemental Specifications S-101 dated 7-12-62; RB-1-55 revised 2-2-59, and Supplemental Specification No. S-307, revised 10-1-64.

DESIGN LOADING - CF 2000 (57).

CONCRETE CLASS C - basic unit stress 1,333 p.s.i.

CONCRETE CLASS E - basic unit stress 1,133 p.s.i.

STRUCTURAL STEEL - ASTM A36-basic unit stress 20,000 p.s.i. (ASTM A7 and A373 steel not permitted).

REINFORCING STEEL - ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 p.s.i. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 p.s.i.

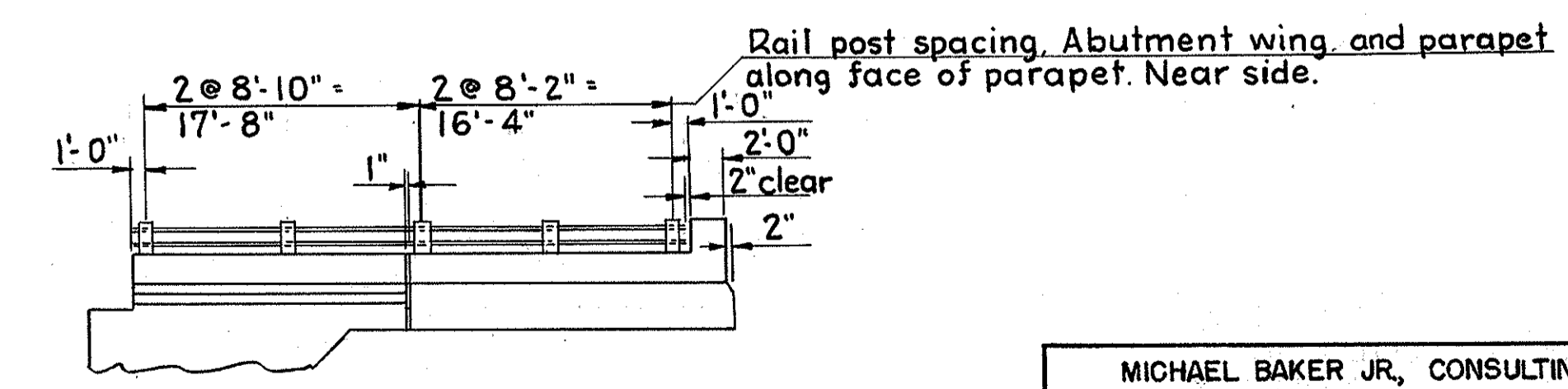
FOUNDATION BEARING PRESSURE: Abutment footings are designed for a maximum bearing pressure of 3.4 tons per square foot, pier footings for 3.3 tons per square foot.

FOOTING KEYS shall be placed against undisturbed earth.

WELDING of structural steel shall be Class "A" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.

SUPERSTRUCTURE GROUND: A No. 1/0 AWG 7 Strand, soft annealed bare copper cable shall be encased in the stem of Abutment B. Lower end of cable to extend to bottom of the abutment footing, and an additional 25' length of cable shall be looped under the footing and separated from the concrete with two (2) layers of tar paper. Extend cable (in one continuous length) through top of abutment seat with lead of sufficient length to exothermic weld upper end of cable to outside girder of superstructure. Payment for electrical grounds is included in the bid for S-25, "Electrical Lighting System Complete."

UTILITY LINES: All expense involved in relocating the affected utility lines shall be borne by the Owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

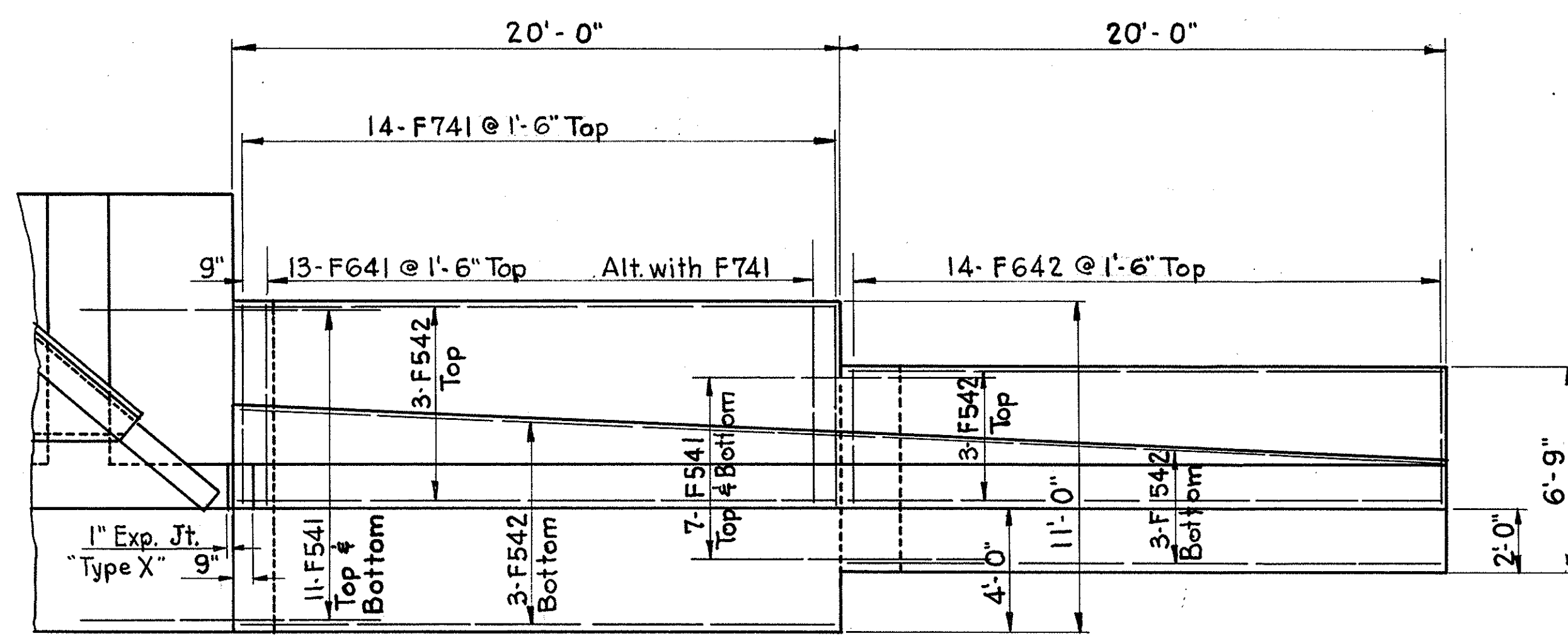


DETAIL "A"

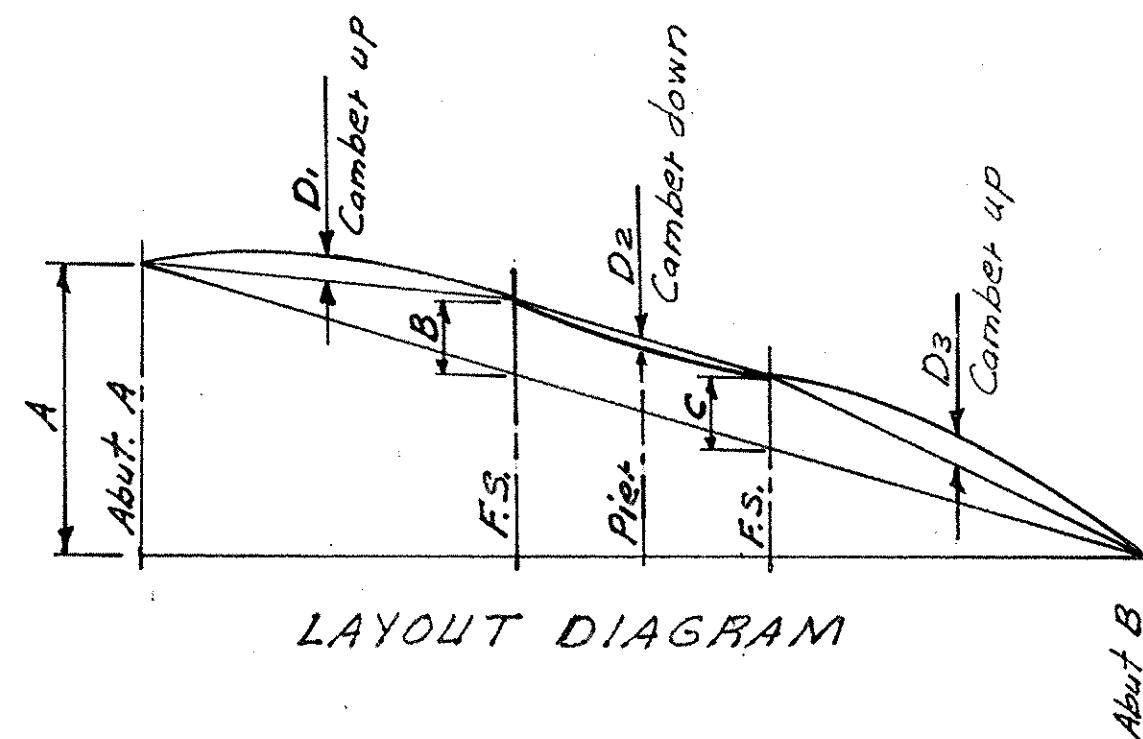
5-7/10 HIGH STRENGTH STEEL BOLTS: High strength bolts that are used as fitting-up bolts may be used in the final assembly if, after all bolts are placed and tightened, the fitting-up are subsequently loosened and retightened by the turn of nut method.

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
GENERAL PLAN & ELEVATION					
BRIDGE NO. MAH-18-1715 UNDER BELLE VISTA AVENUE					
MAHONING COUNTY					STA. 606+54.21
Designed	Drawn	Traced	Checked	Reviewed Date	Revised
EFR	EJ	R.T.R.	R.L.S.	10-14-63	

**MAHONING COUNTY
MAH-18-15.50**



PLAN



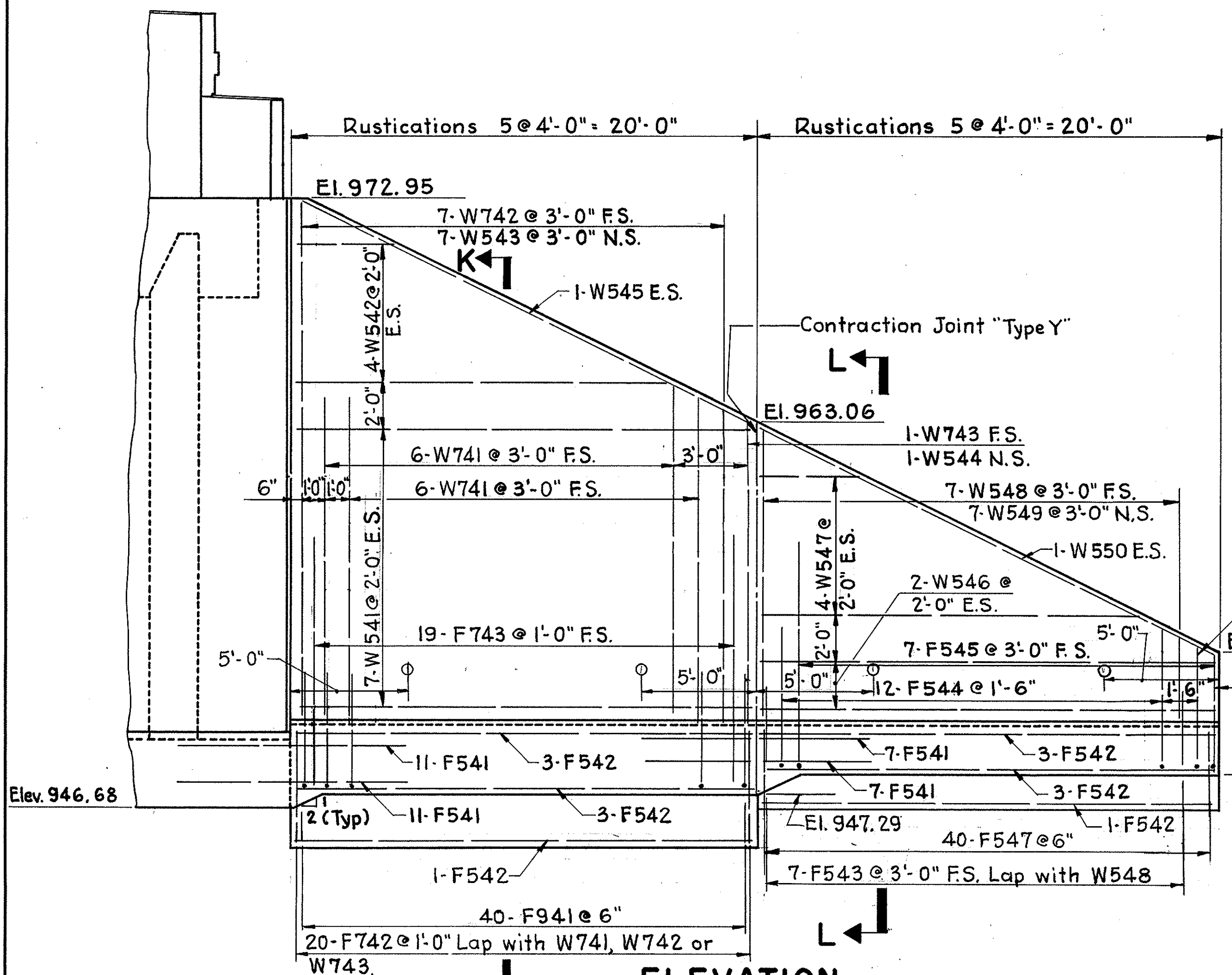
LAYOUT DIAGRAM

DEFLECTIONS TO BE USED FOR SETTING BULB ANGLE GUTTERS

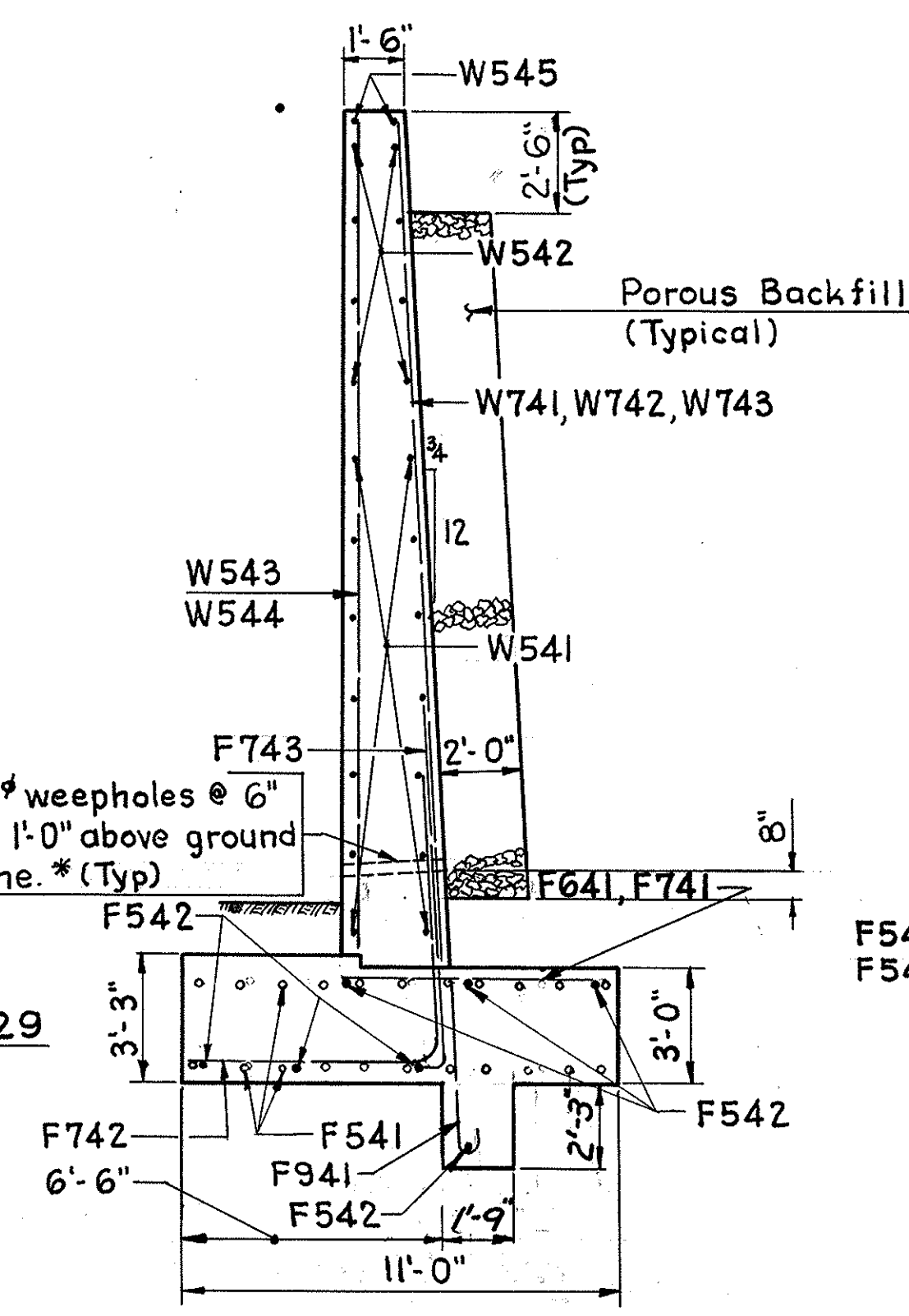
Spans 1 & 2. Points →	1/4	1/2	3/4
Deflection due to Steel	3/16	1/8	3/16
Defl. due to Remaining D.L.	1/16	1/16	1/8
Total D.L. Deflection	1/8	1/8	1/4

DATA FOR CAMBERING GIRDERS

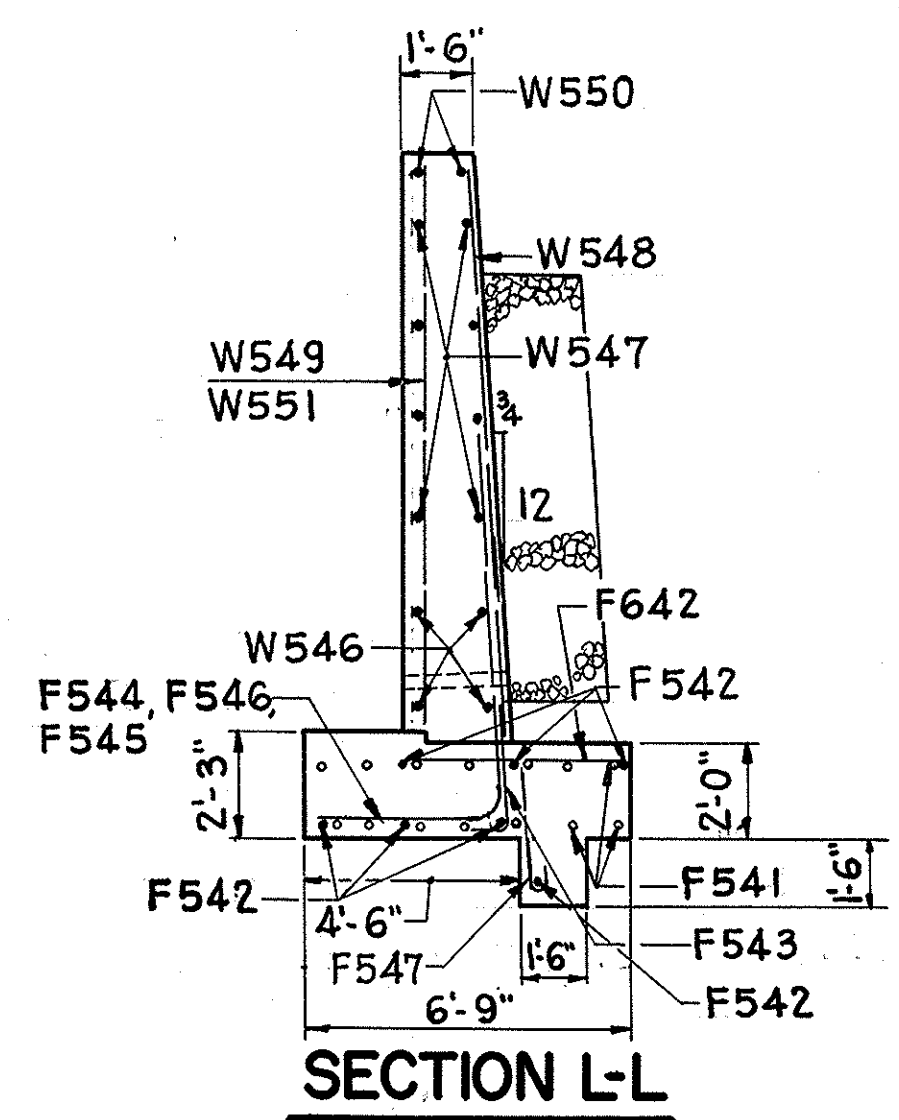
Girder	Camber @ mid-point					
	A	B	C	D ₁	D ₂	D ₃
6	6'-2 1/4"	4 1/2"	3 7/8"	1 3/4"	1 1/4"	1 1/8"
5	6'-0 1/2"	4 3/8"	4 1/8"	1 3/8"	1 1/8"	1 1/8"
4	5'-11 1/2"	5 1/2"	4 7/8"	1 3/8"	1 1/8"	1"
3	5'-9 3/4"	5 1/4"	5 1/4"	1 3/8"	1 3/8"	1 3/8"
2	5'-7 3/4"	5 1/2"	5 1/2"	1 3/8"	1 3/8"	1 3/8"
1	5'-5 1/2"	5 3/8"	5 1/4"	1 3/8"	1 3/8"	1 1/8"



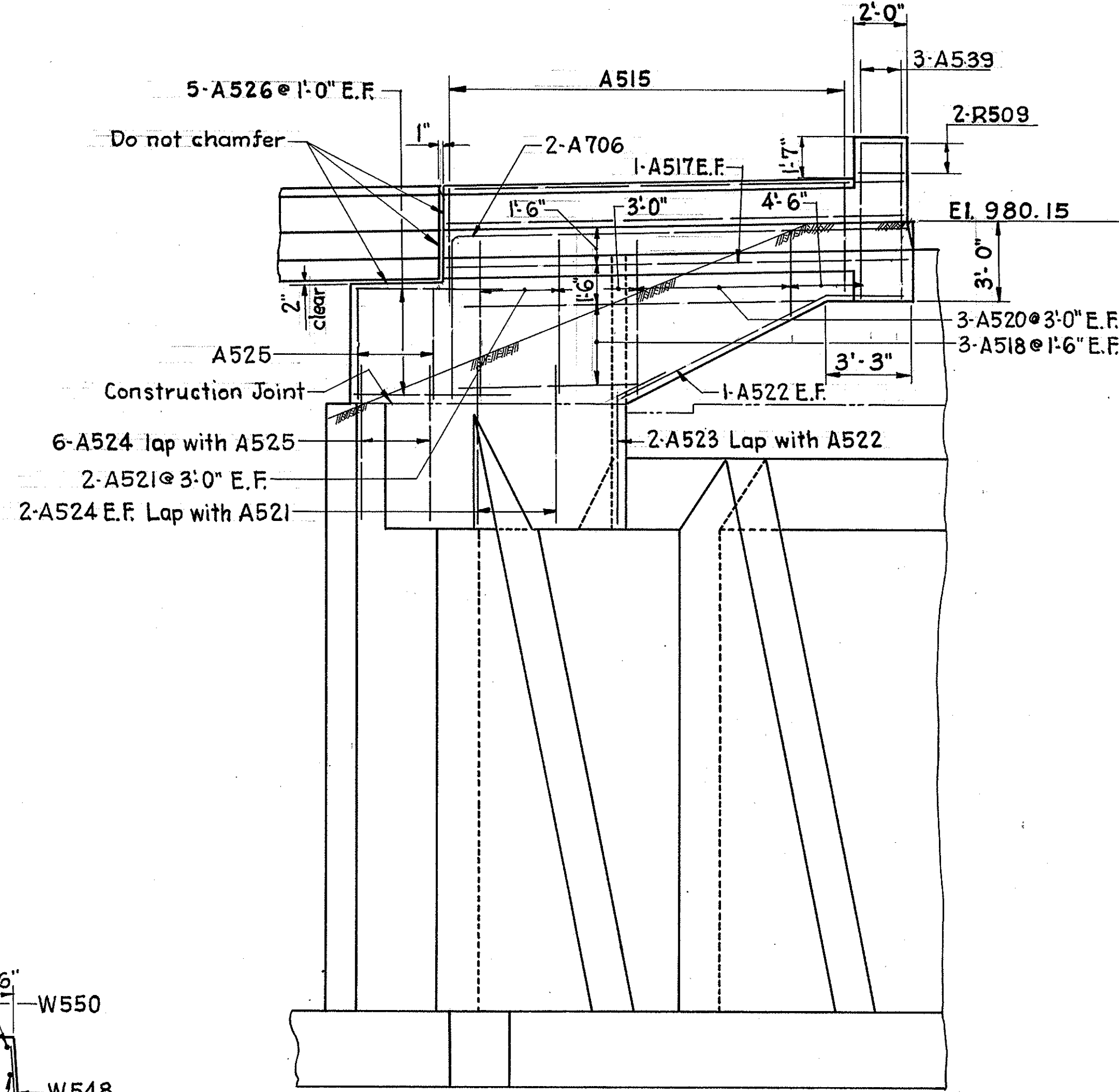
ELEVATION



SECTION K-K



SECTION L-L



VIEW D-D

See Abutment 'A' Sheet

LEGEND

- E.S. = Each Side
- F.S. = Far Side
- N.S. = Near Side

* Locate weepholes as shown on Elevation.

NOTE:

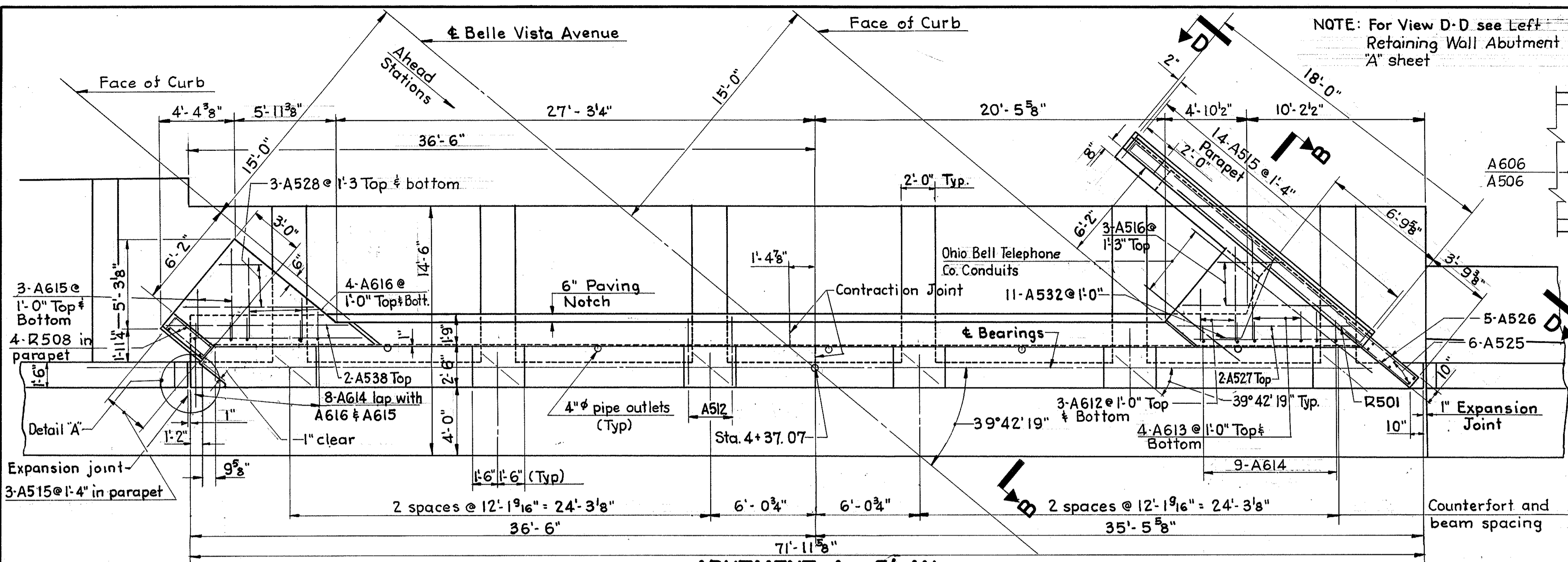
For joint and rustication groove details see Left Retaining Wall Abutment 'B'.
Porous backfill, 2 ft. thick, full length of abutments and wings shall extend up to the underside of the approach slab or within 2'-6" of the top of the wings.

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

**LEFT RETAINING WALL
ABUTMENT "A"**
BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

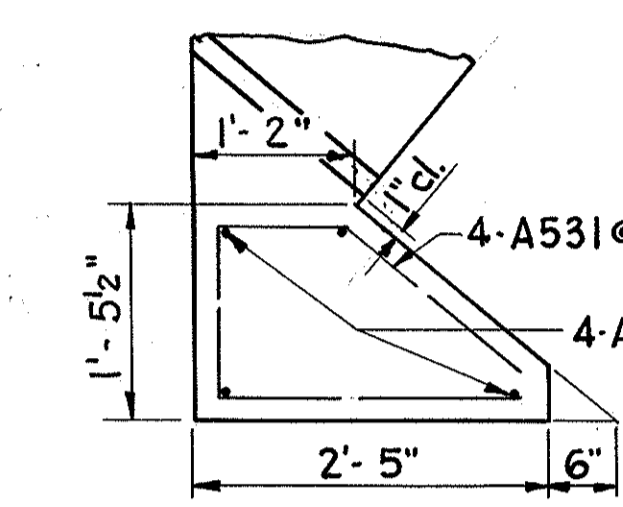
MAHONING CO.				STA. 606+54.21	
Designed	Drawn	Traced	Checked	Reviewed	Date
RLS	EFR	P.T.R.	EFR	F.K.	2-23-66
					10-14-63

MAHONING COUNTY
MAH-18-15.50

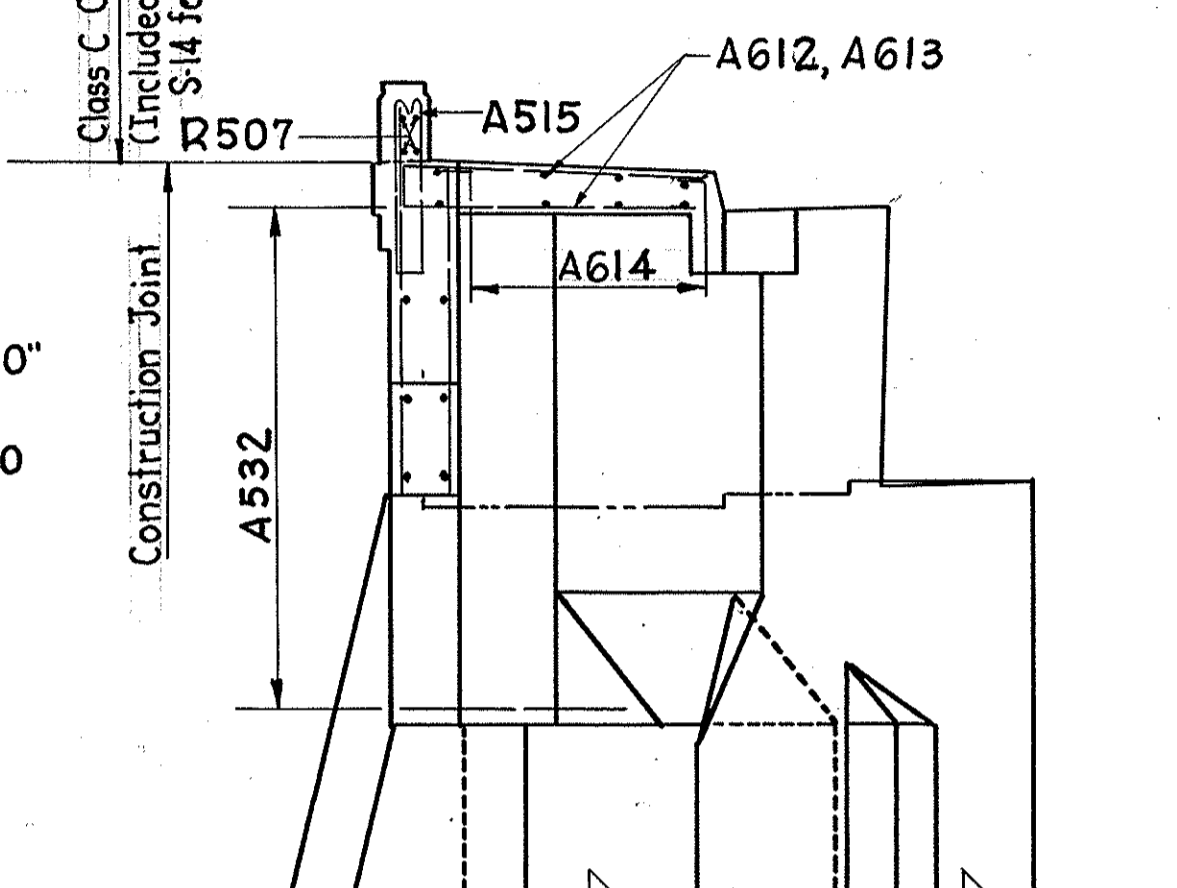


ABUTMENT A - PLAN

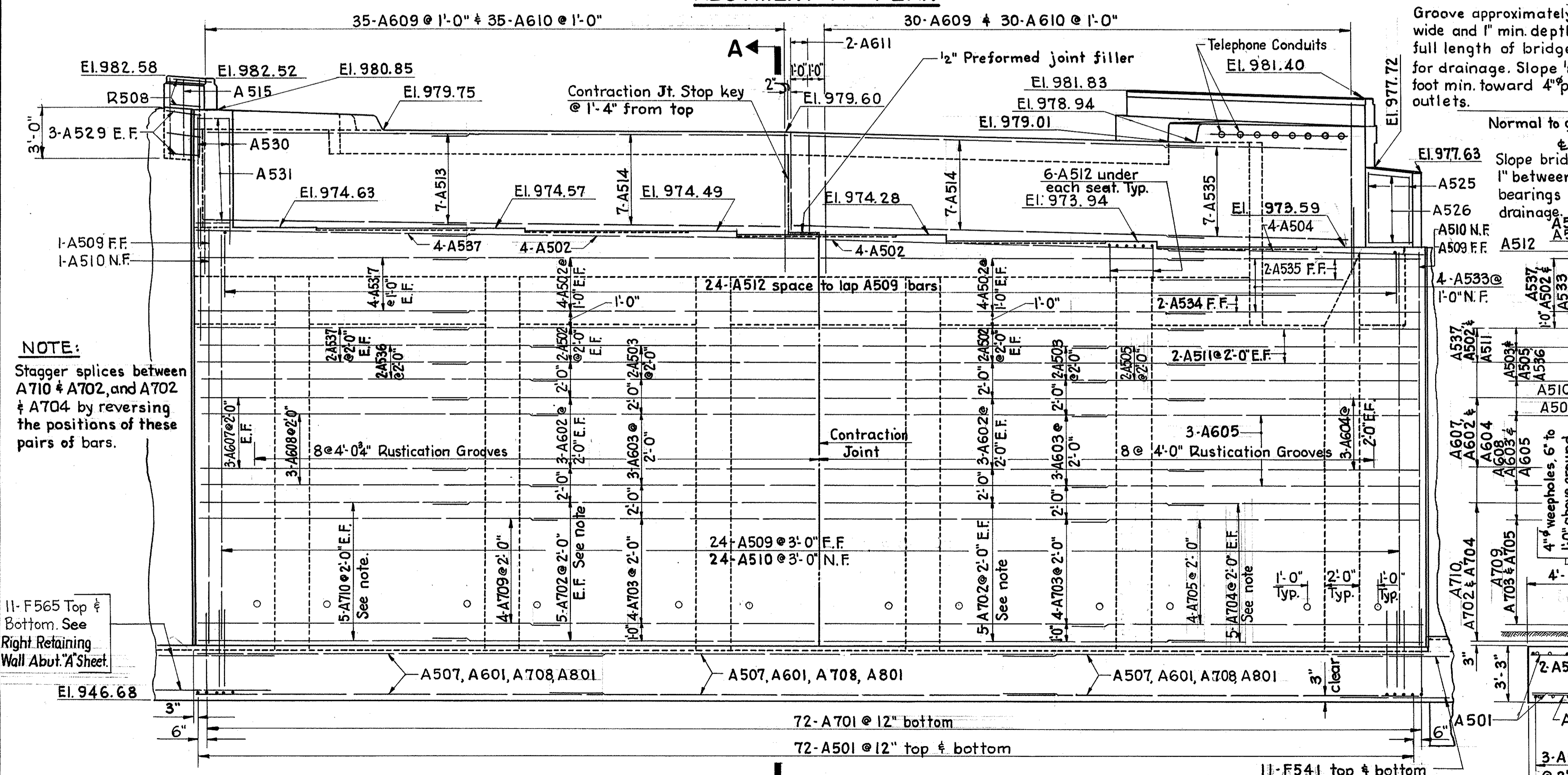
SECTION C-C



DETAIL "A"

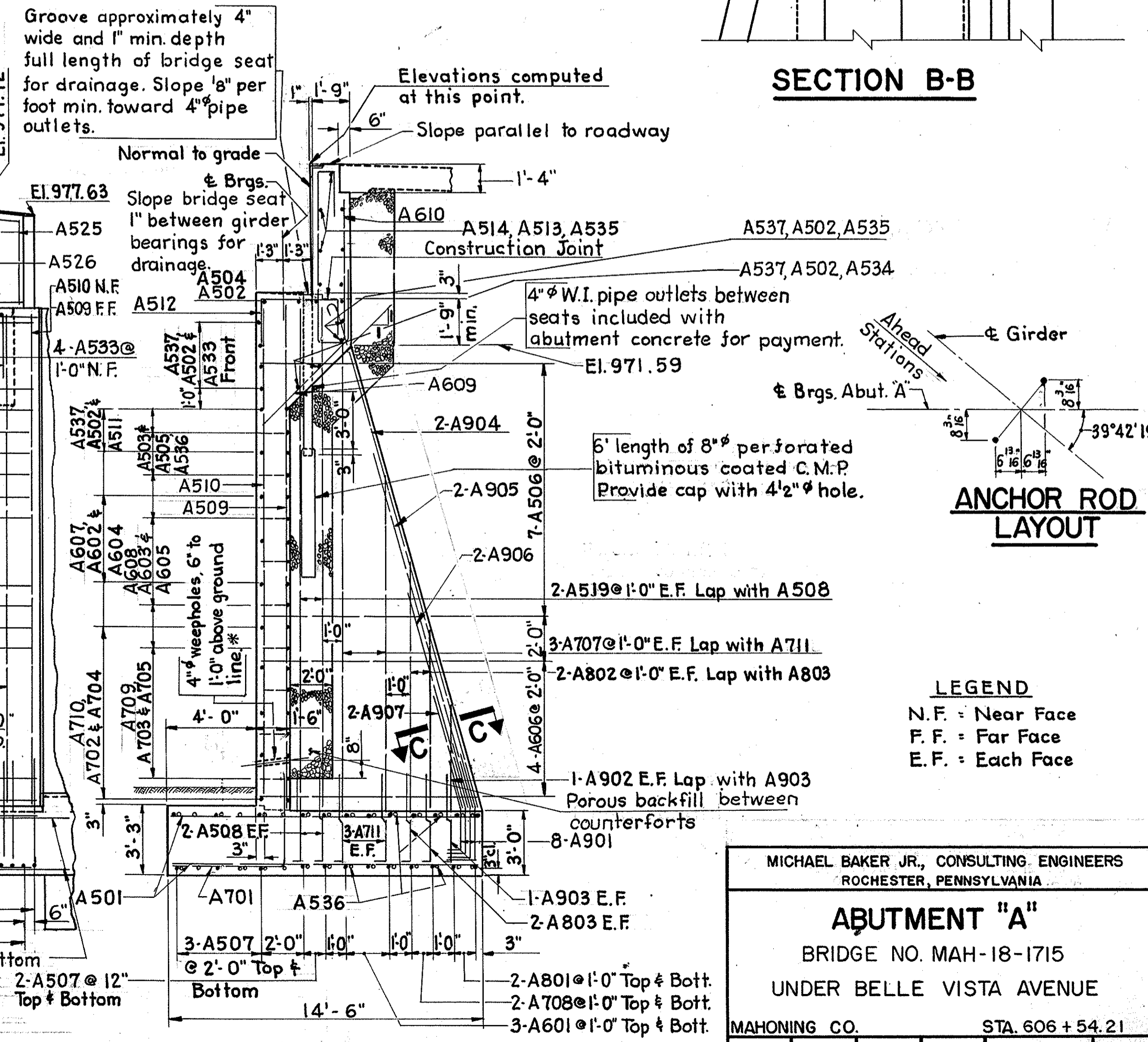


SECTION B-B



ELEVATION

SECTION A-A



ANCHOR ROD LAYOUT

NOTE:
Stagger splices between A710 & A702, and A702 & A704 by reversing the positions of these pairs of bars.

NOTE:
For joint and rustication groove details see Left Retaining Wall Abutment "B".

11-F541 top & bottom
See Left Retaining Wall Abutment "A" Sheet
2-A507 @ 12" Top & Bottom

* Locate weepholes as shown in Elevation.

LEGEND
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face

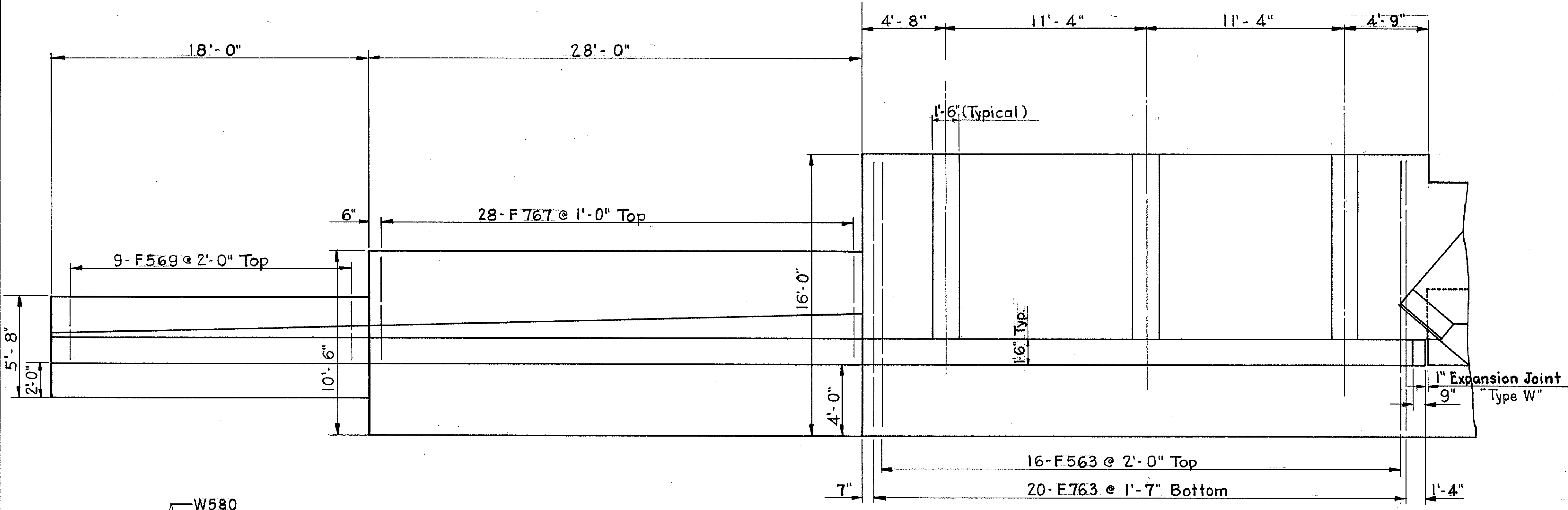
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

ABUTMENT "A"
BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

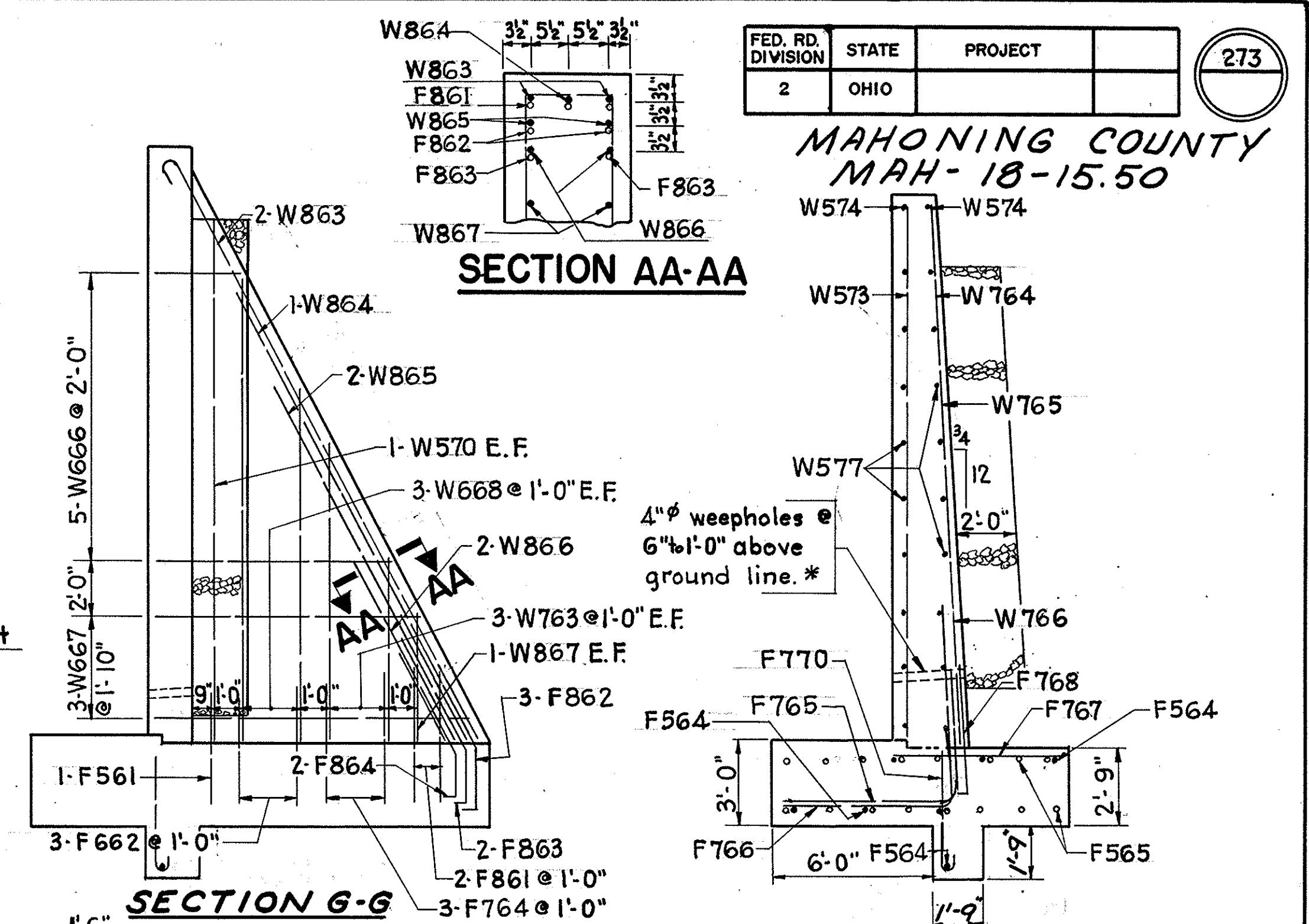
Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
DM	EFR	P.T.R.	EFR	F.K.	10-14-63	

MAHONING CO. STA. 606 + 54.21

**MAHONING COUNTY
MAH-18-15.50**

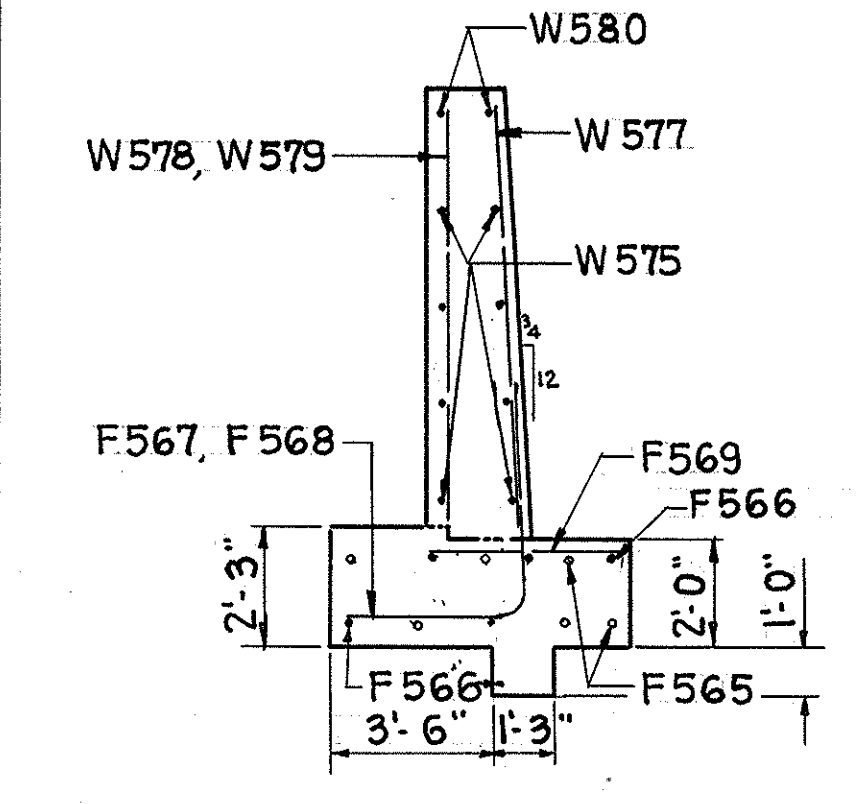


PLAN

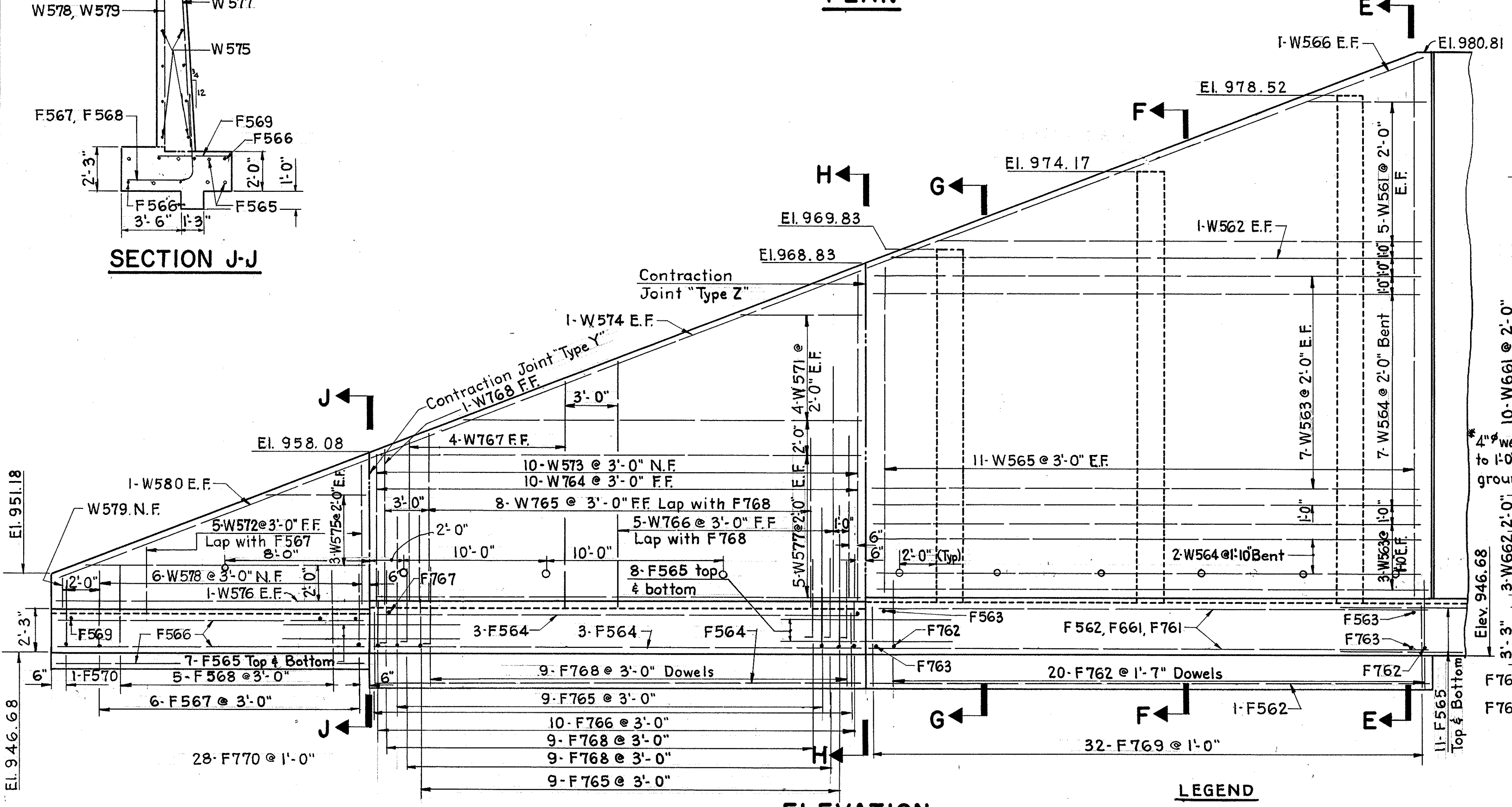


SECTION AA-AA

SECTION G-G

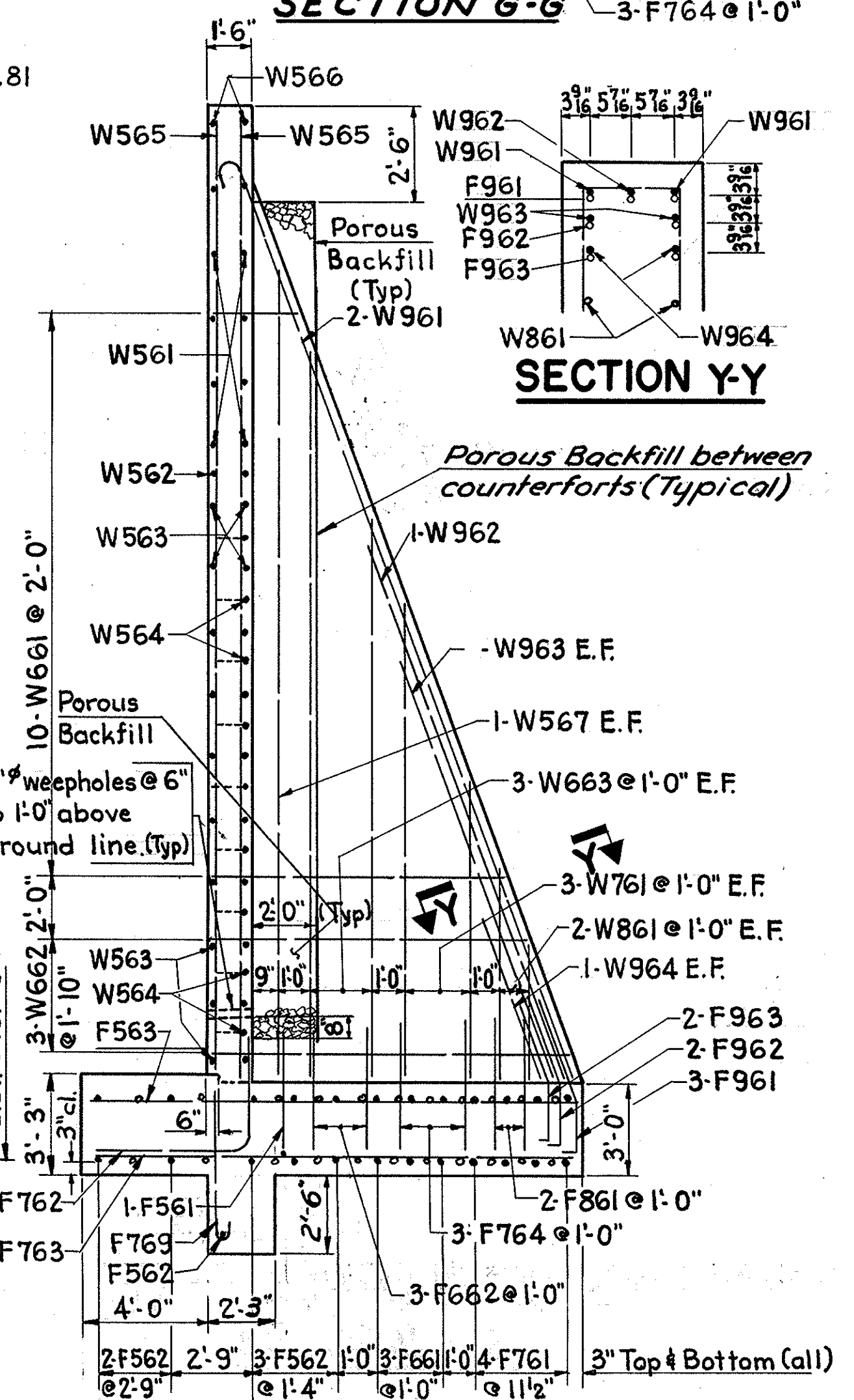


SECTION J-J

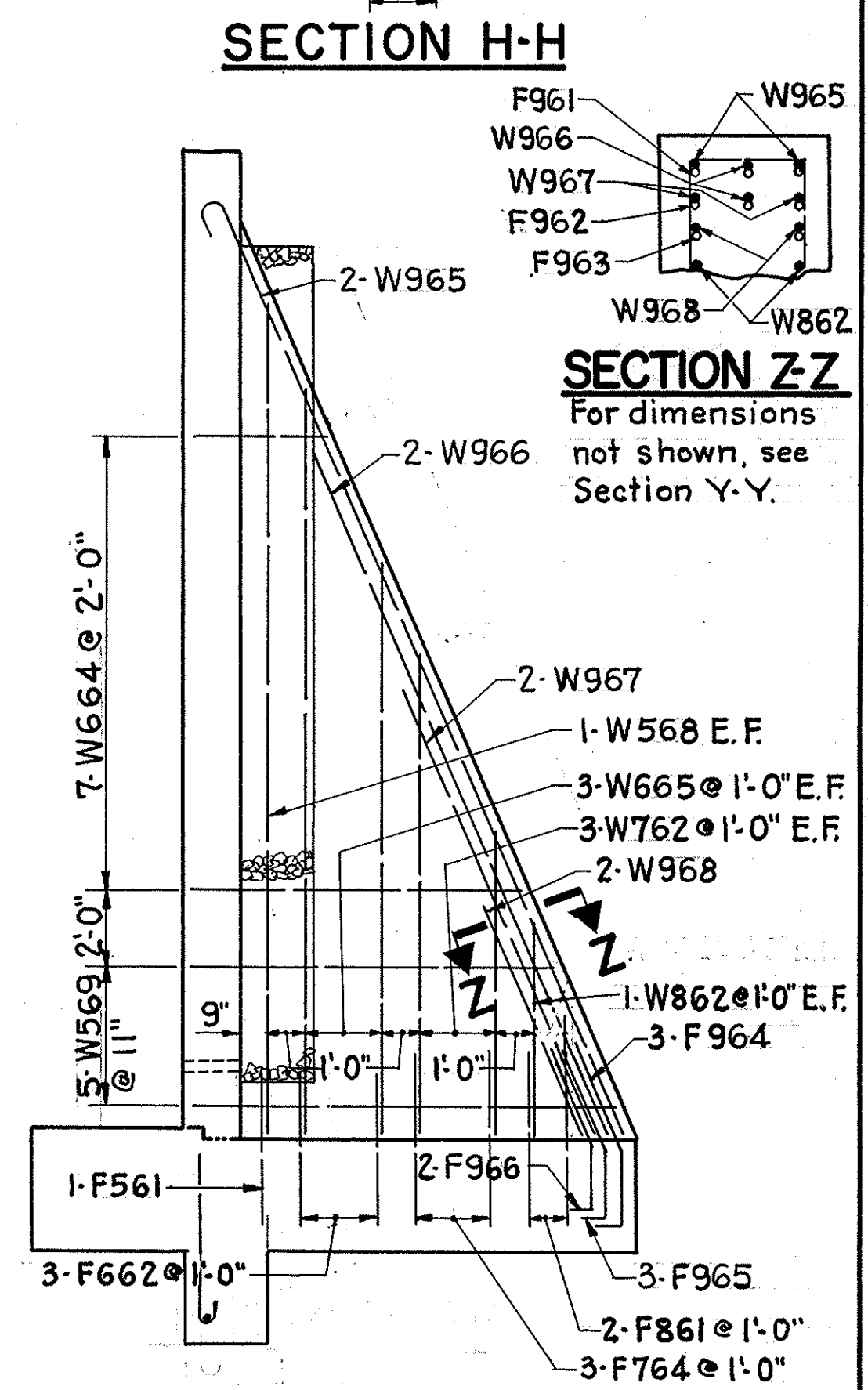


ELEVATION

LEGEND
 N.F. = Near Face
 F.F. = Far Face
 E.F. = Each Face



SECTION E-E



SECTION H-H

SECTION Z-Z
 For dimensions not shown, see Section Y-Y.

NOTE:
 For joint and rustication groove details see Left Retaining Wall Abutment 'B'.

* Locate weepholes as shown in Elevation.

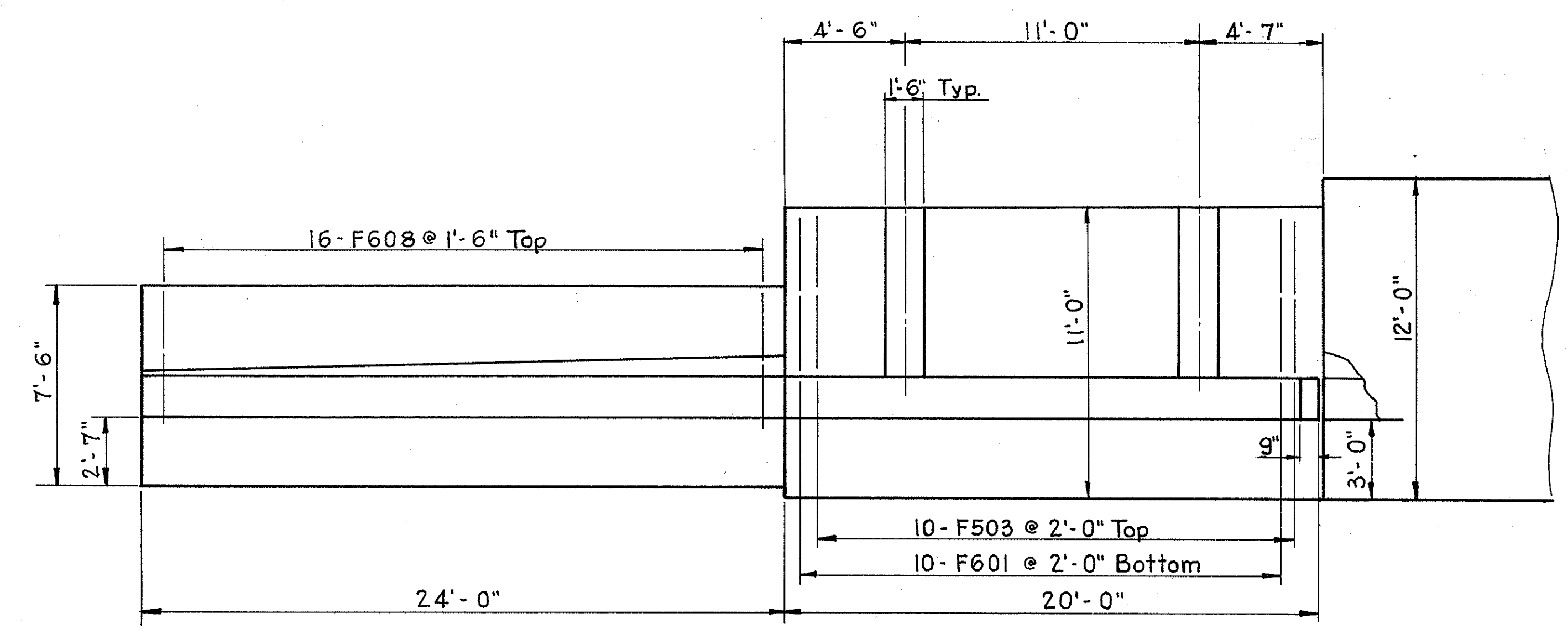
SECTION F-F

MICHAEL BAKER JR., CONSULTING ENGINEERS
 ROCHESTER, PENNSYLVANIA

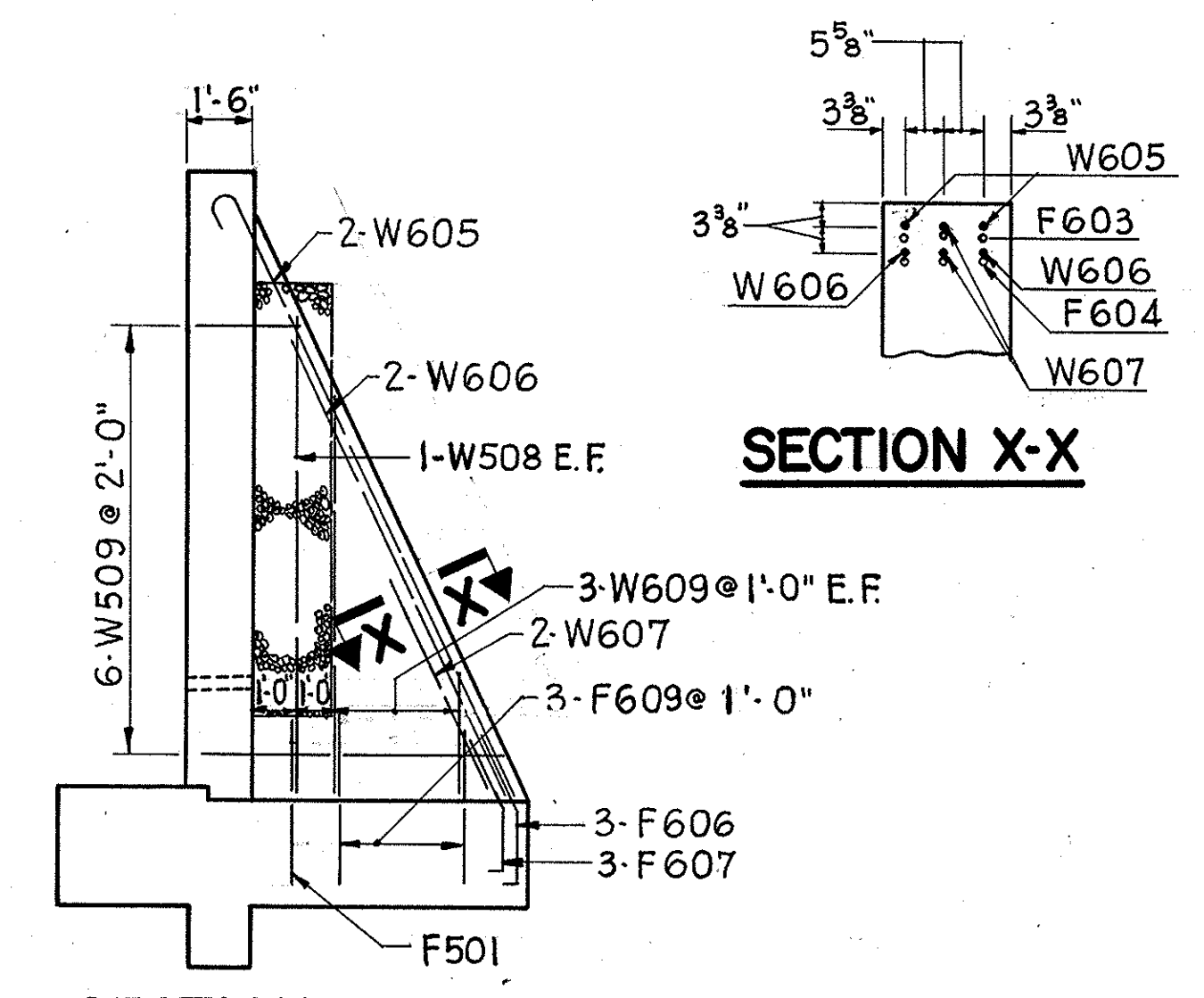
**RIGHT RETAINING WALL
 ABUTMENT "A"**
 BRIDGE NO. MAH-18-1715
 UNDER BELLE VISTA AVENUE

MAHONING CO.	STA. 606 + 54.21				
Designed	Drawn	Traced	Checked	Reviewed	Date
RLS	EFR	P.T.R.	EFR	E.K.	10-14-63

MAHONING COUNTY
MAH-18-15.50

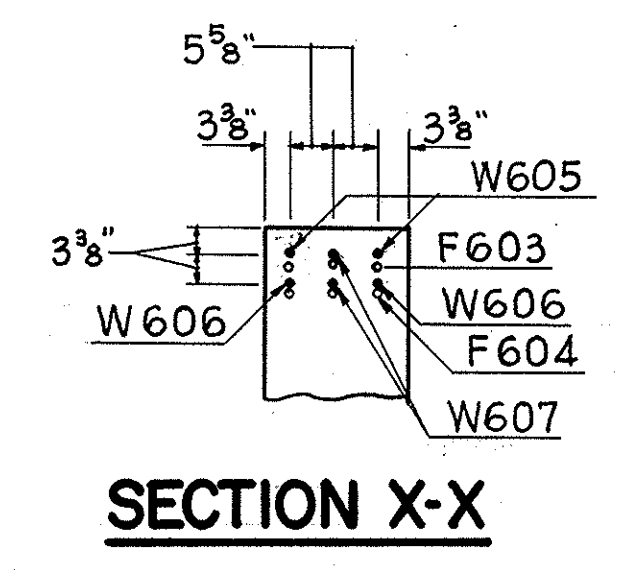


PLAN

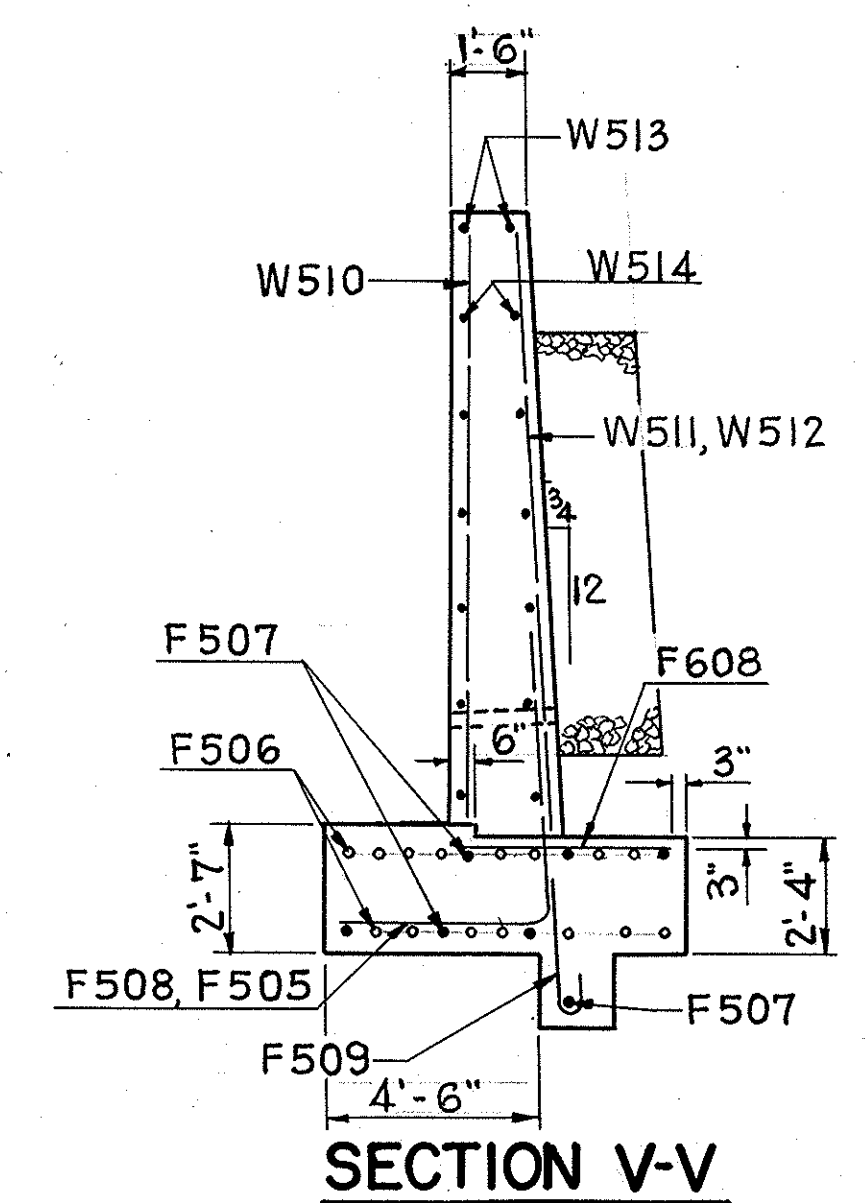


SECTION U-U

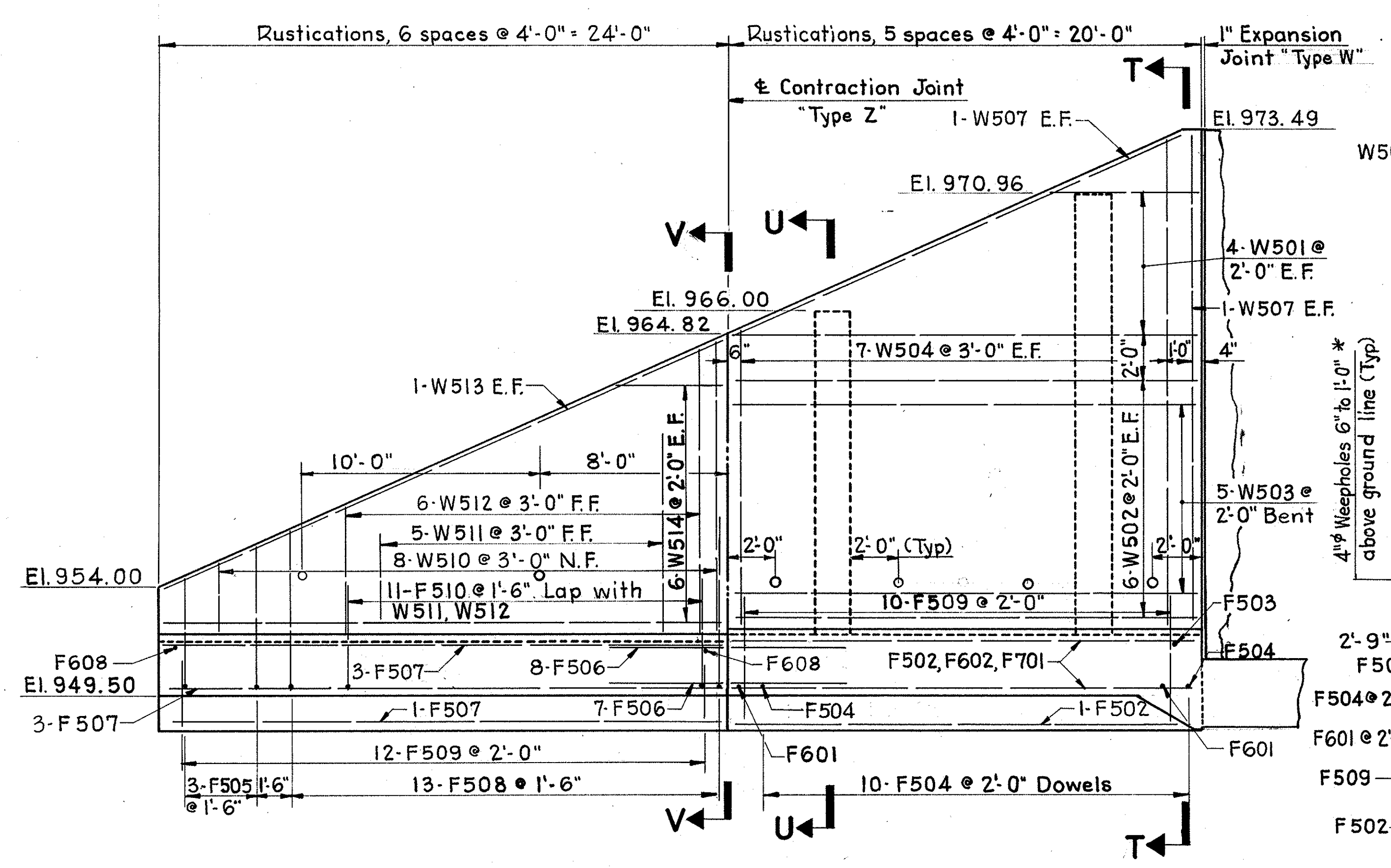
Similar to Section T-T except as noted.



SECTION X-X

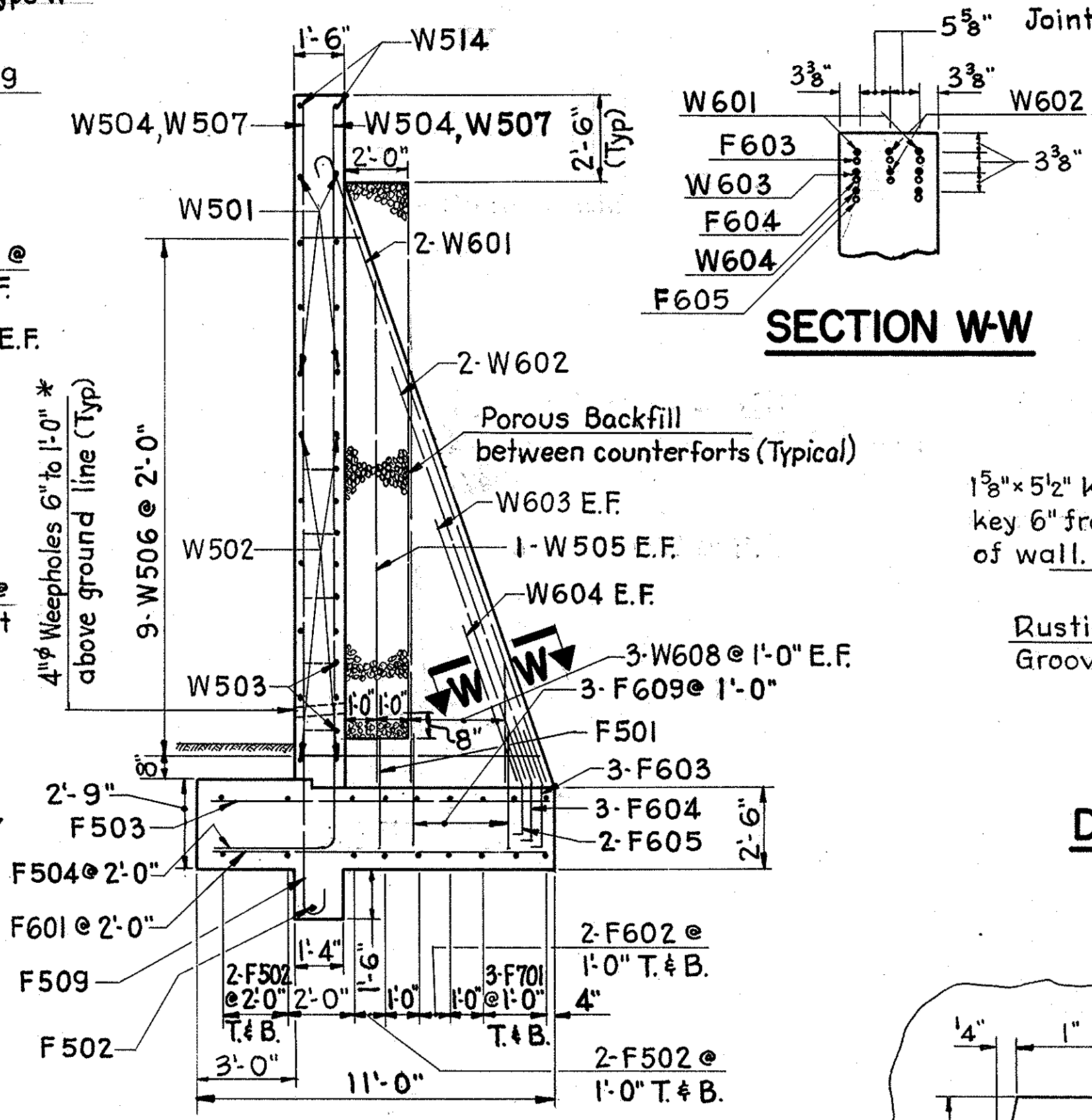


SECTION V-V

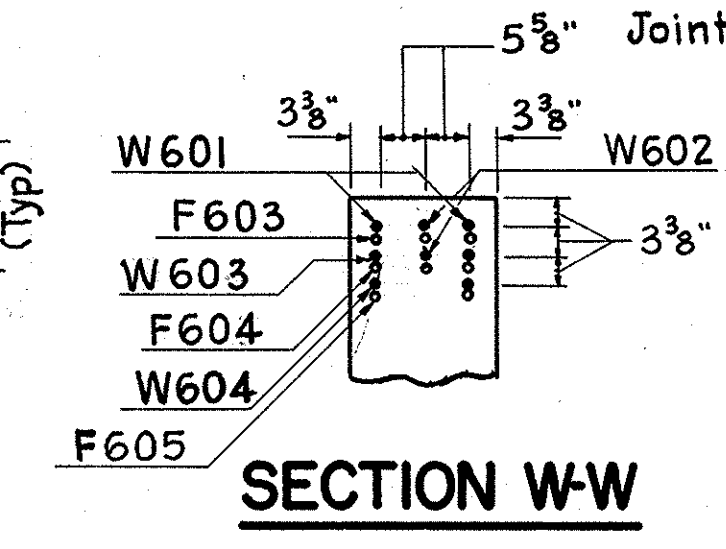


ELEVATION

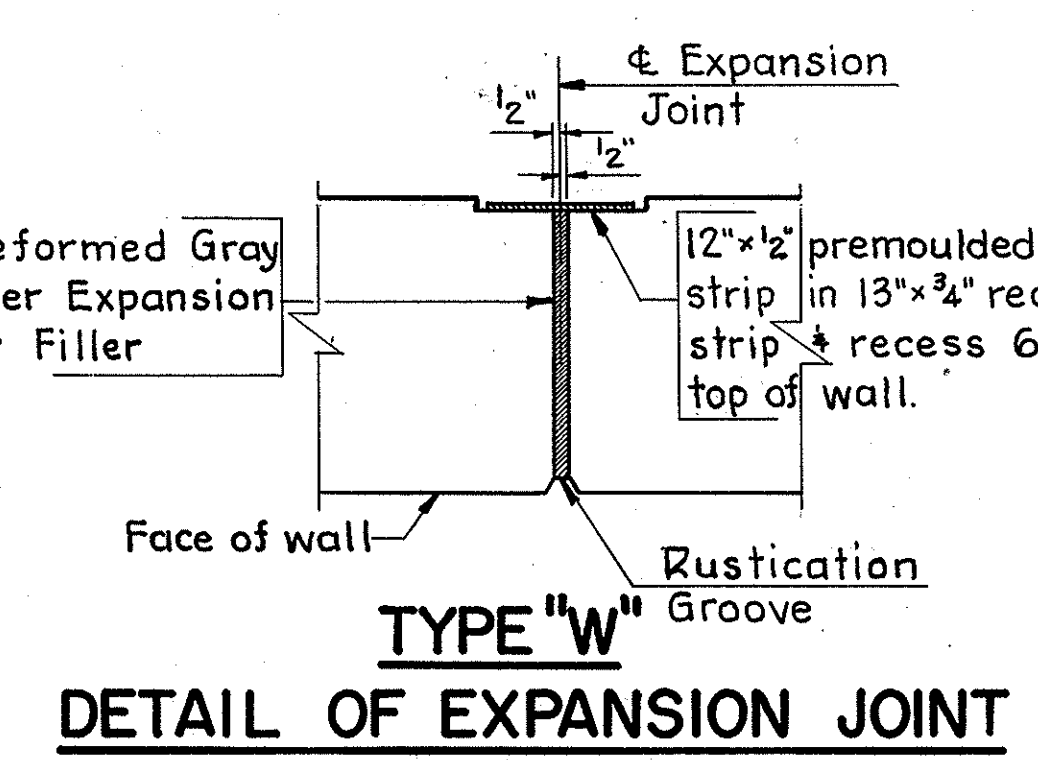
* Locate weepholes as shown in elevation.



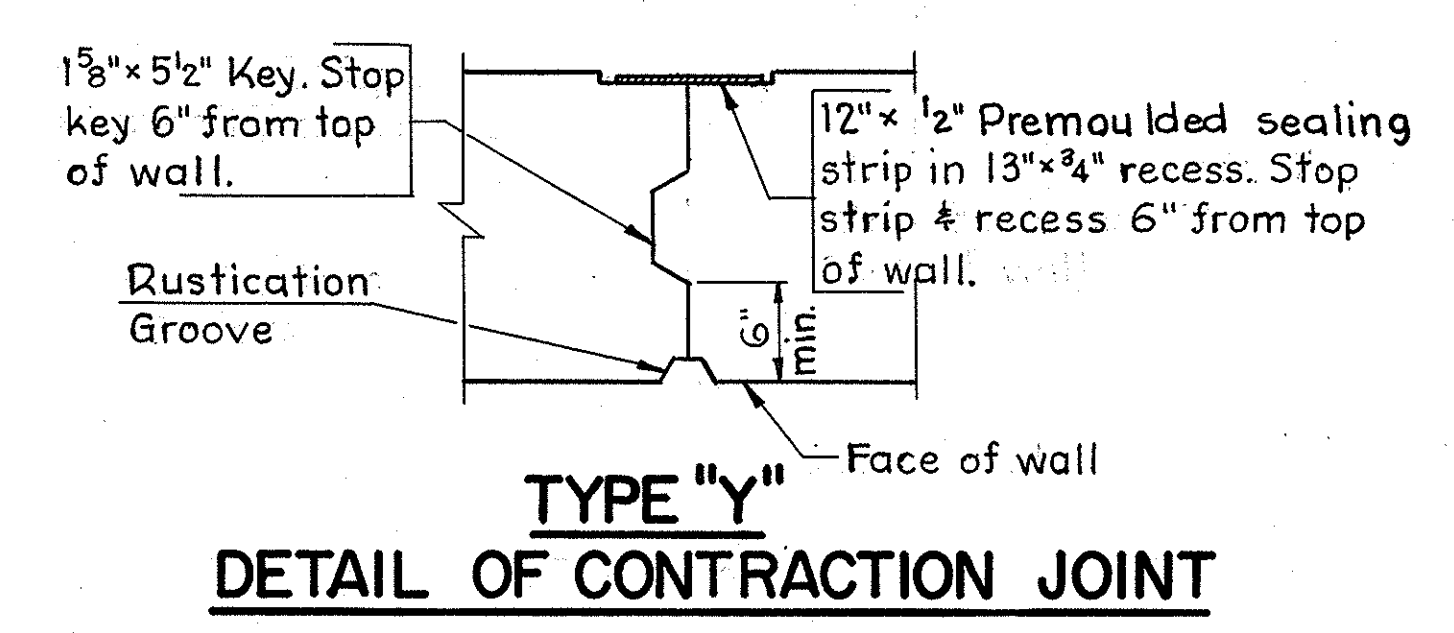
SECTION T-T



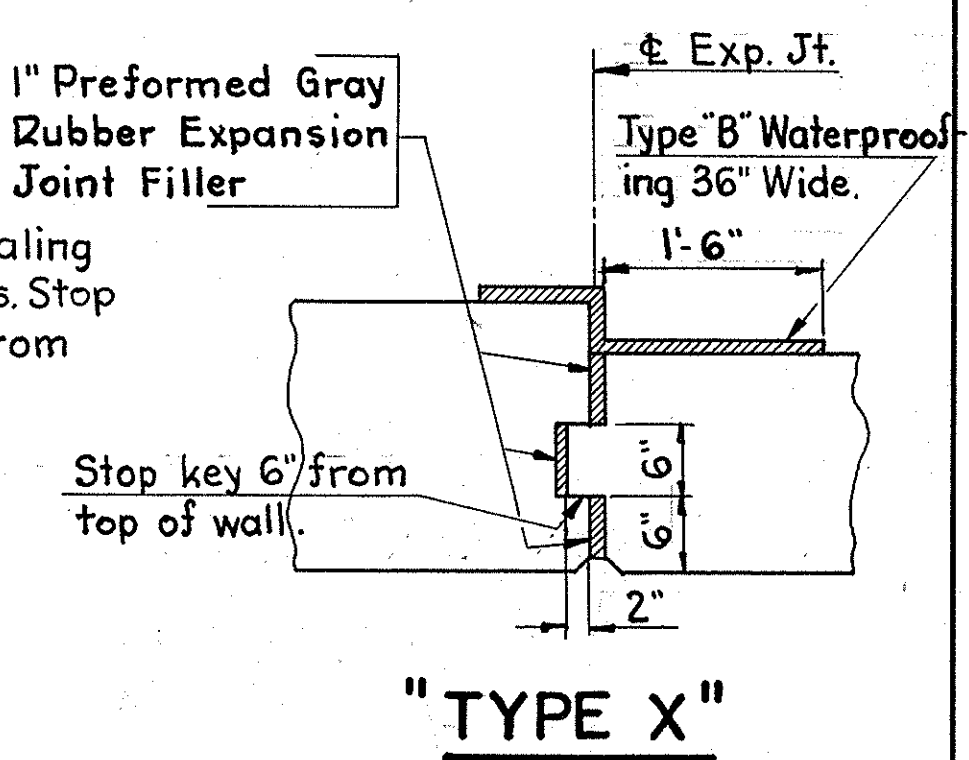
SECTION W-W



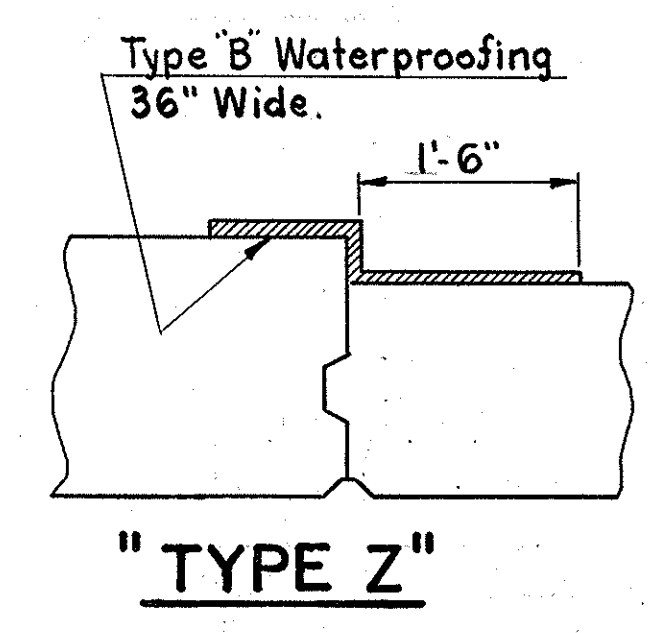
DETAIL OF EXPANSION JOINT TYPE "W"



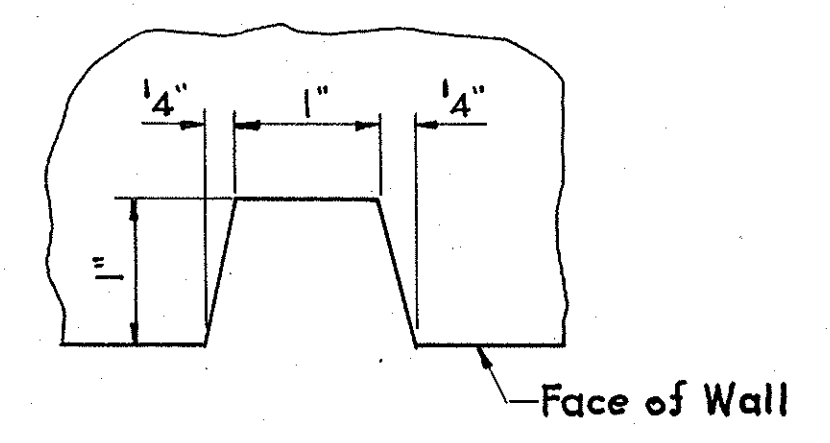
DETAIL OF CONTRACTION JOINT TYPE "Y"



DETAIL OF EXPANSION JOINT TYPE "X"



DETAIL OF CONTRACTION JOINT TYPE "Z"

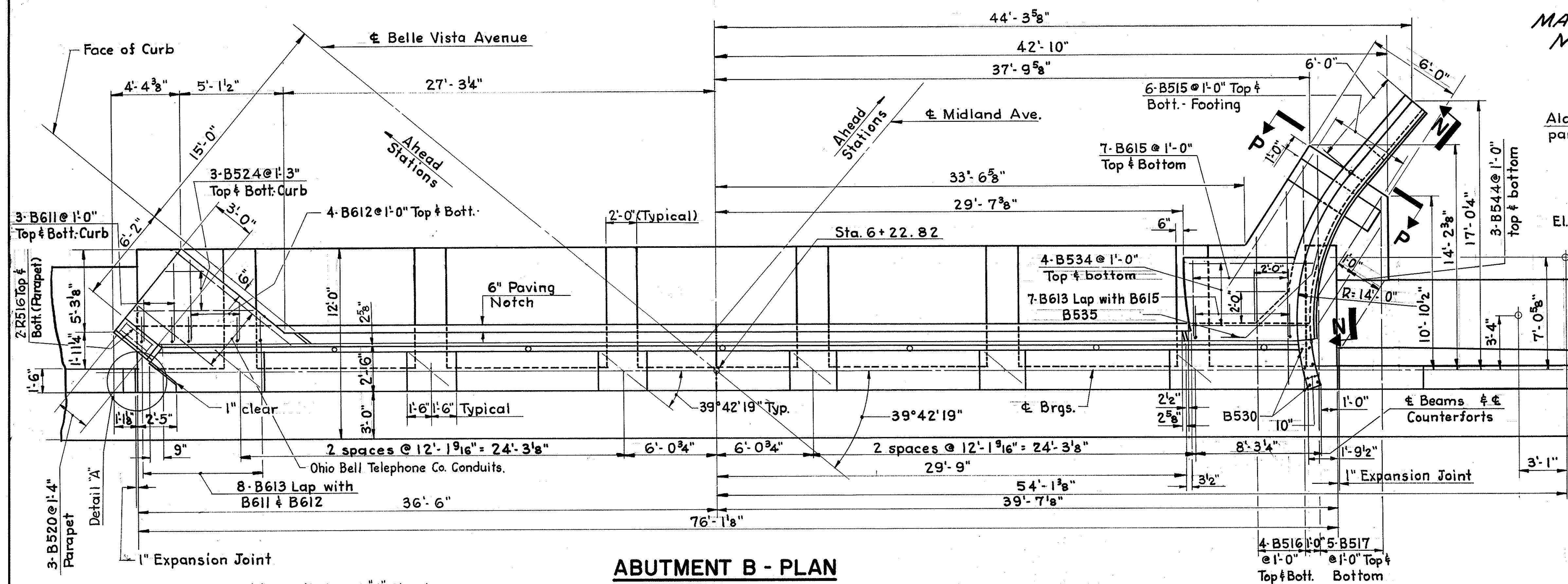


DETAIL OF RUSTICATION

LEGEND
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face
T.&B. = Top & Bottom

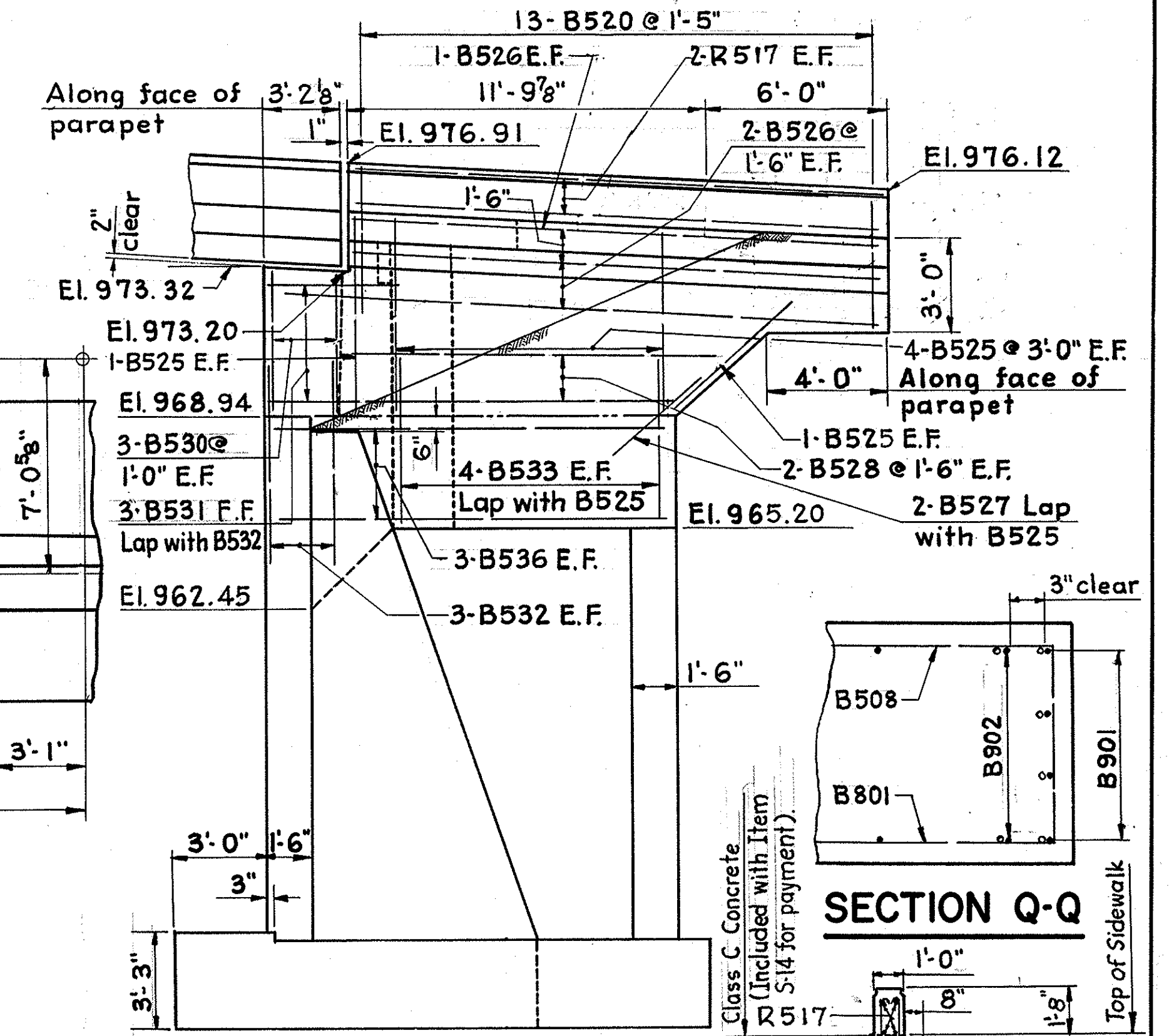
MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
LEFT RETAINING WALL ABUTMENT "B"					
BRIDGE NO. MAH-18-1715					
UNDER BELLE VISTA AVENUE					
MAHONING CO.			STA. 606 + 54.21		
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE
RLS	EFR	R.T.R.	EFR	E.K.	10-14-63

FEDERAL RD DIVISION	STATE	PROJECT
2	OHIO	

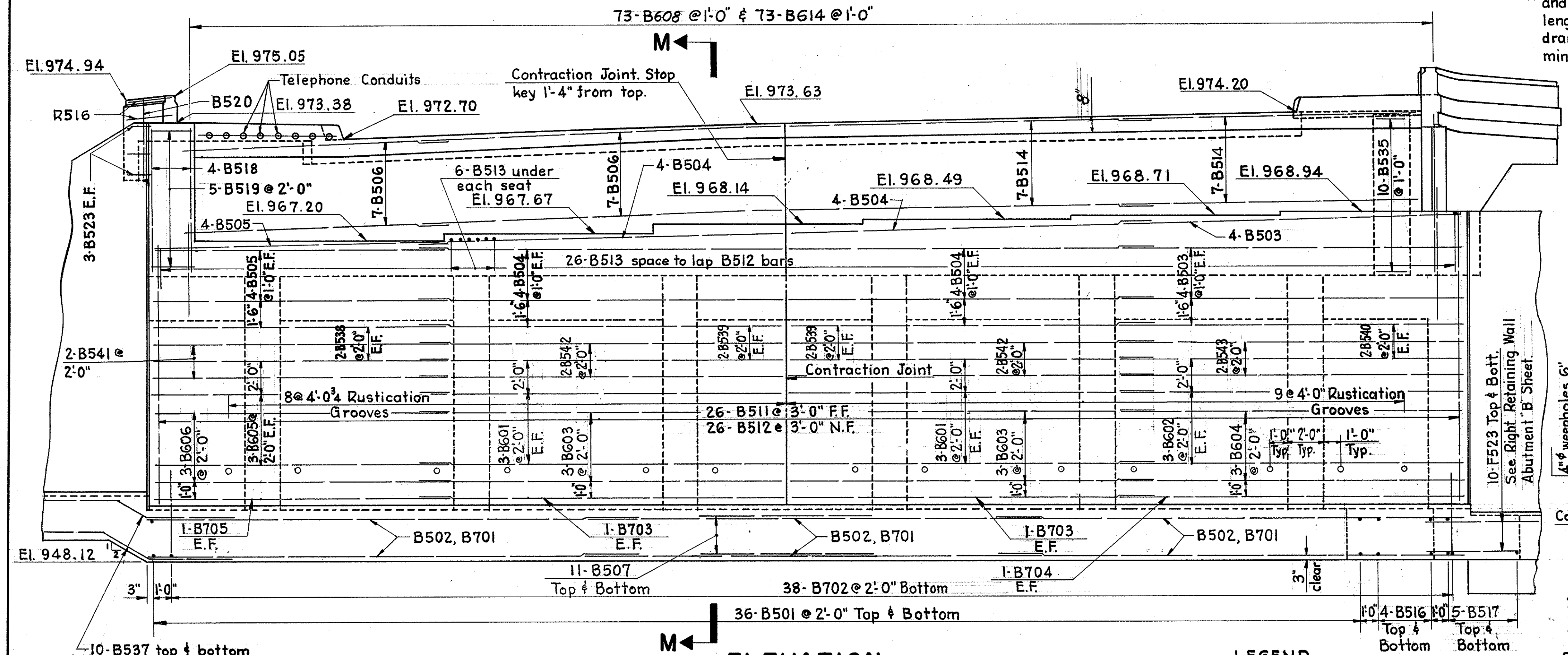


ABUTMENT B - PLAN

NOTE: For Detail "A" see Abutment "A" Sheet.



SECTION Q-Q



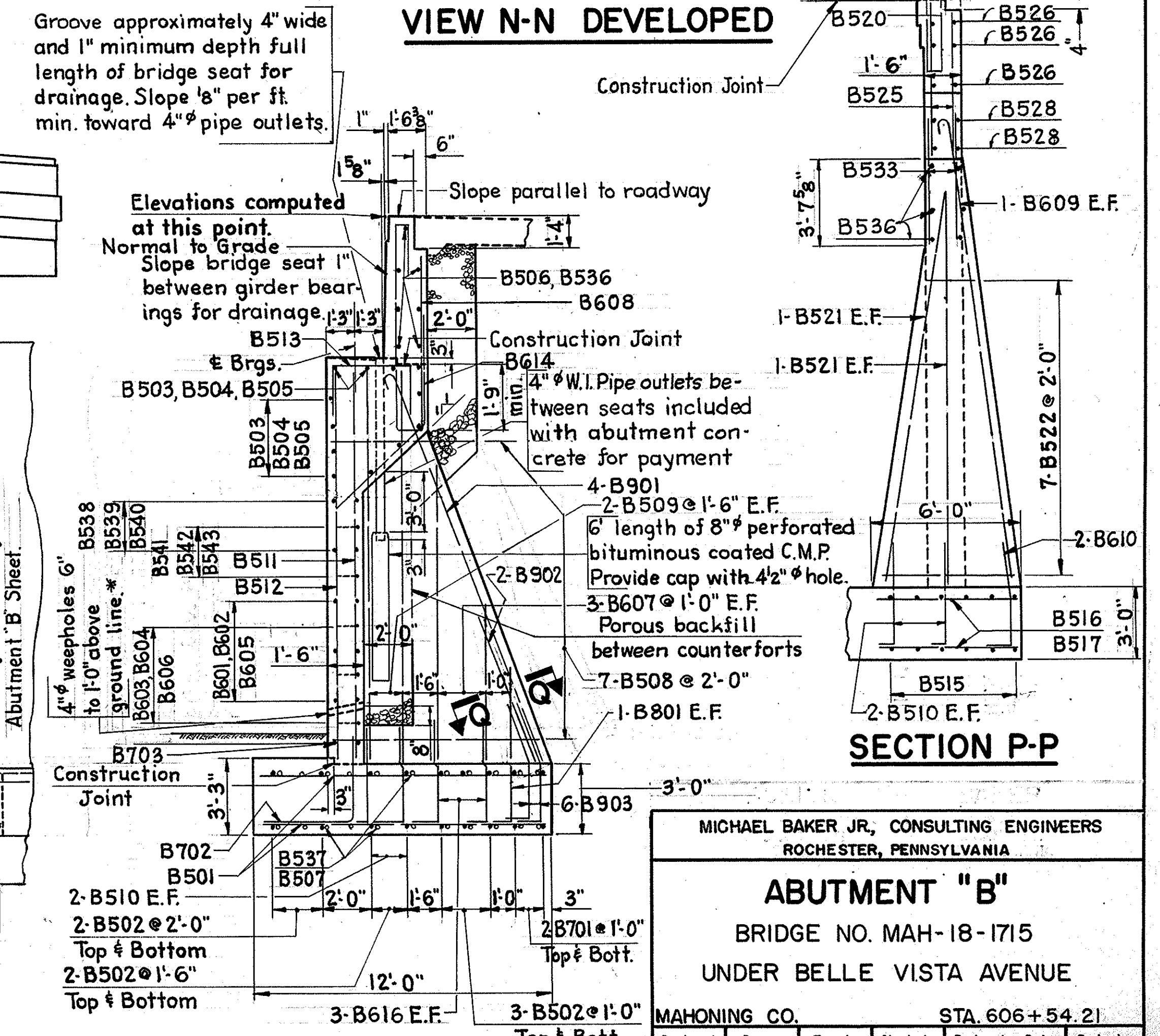
ELEVATION

NOTE:
• For joint and rustication groove details see Left Retaining Wall Abutment "B"
• For ground details, see General Notes on Gen. Plan & Elev. Sheet.

NOTE:
For Anchor Rod Layout see Right Retaining Wall Abutment "B" Sheet.

LEGEND
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face

* Locate weepholes as shown in Elevation.



VIEW N-N DEVELOPED

SECTION P-P

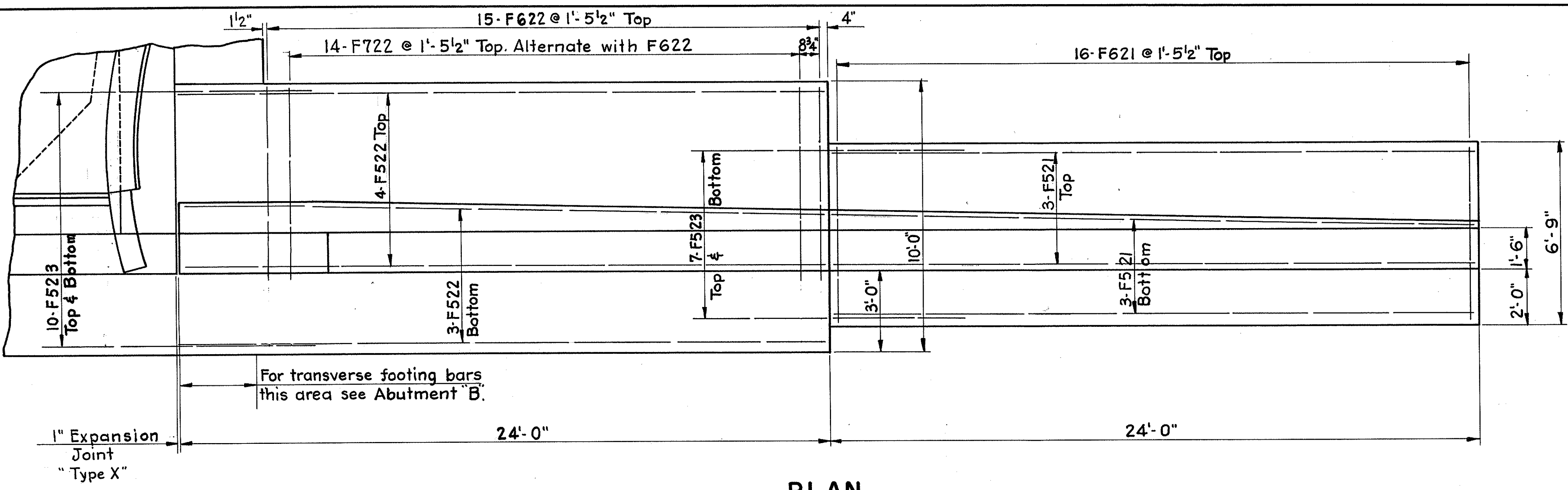
SECTION M-M

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENT "B"					
BRIDGE NO. MAH-18-1715 UNDER BELLE VISTA AVENUE					
MAHONING CO.			STA. 606 + 54.21		
Designed	Drawn	Traced	Checked	Reviewed	Date
EFR	EFR	P.T.R.	RLS	F.K.	10-14-63

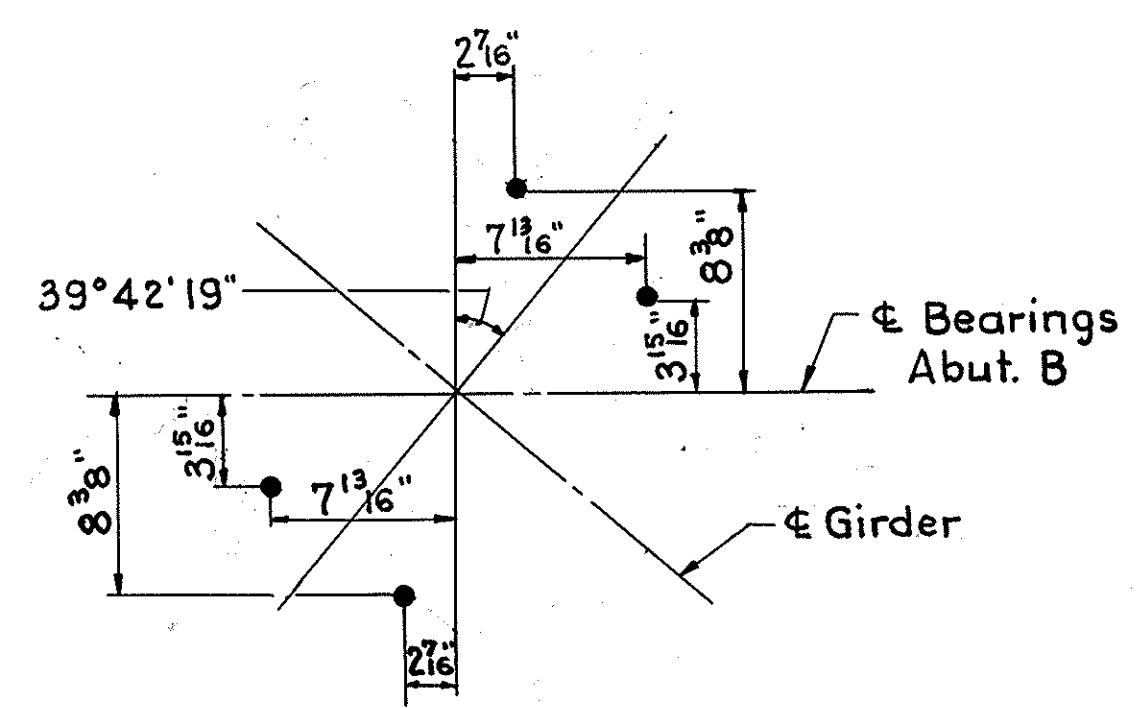
FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

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MAHONING COUNTY
MAH-18-15.50

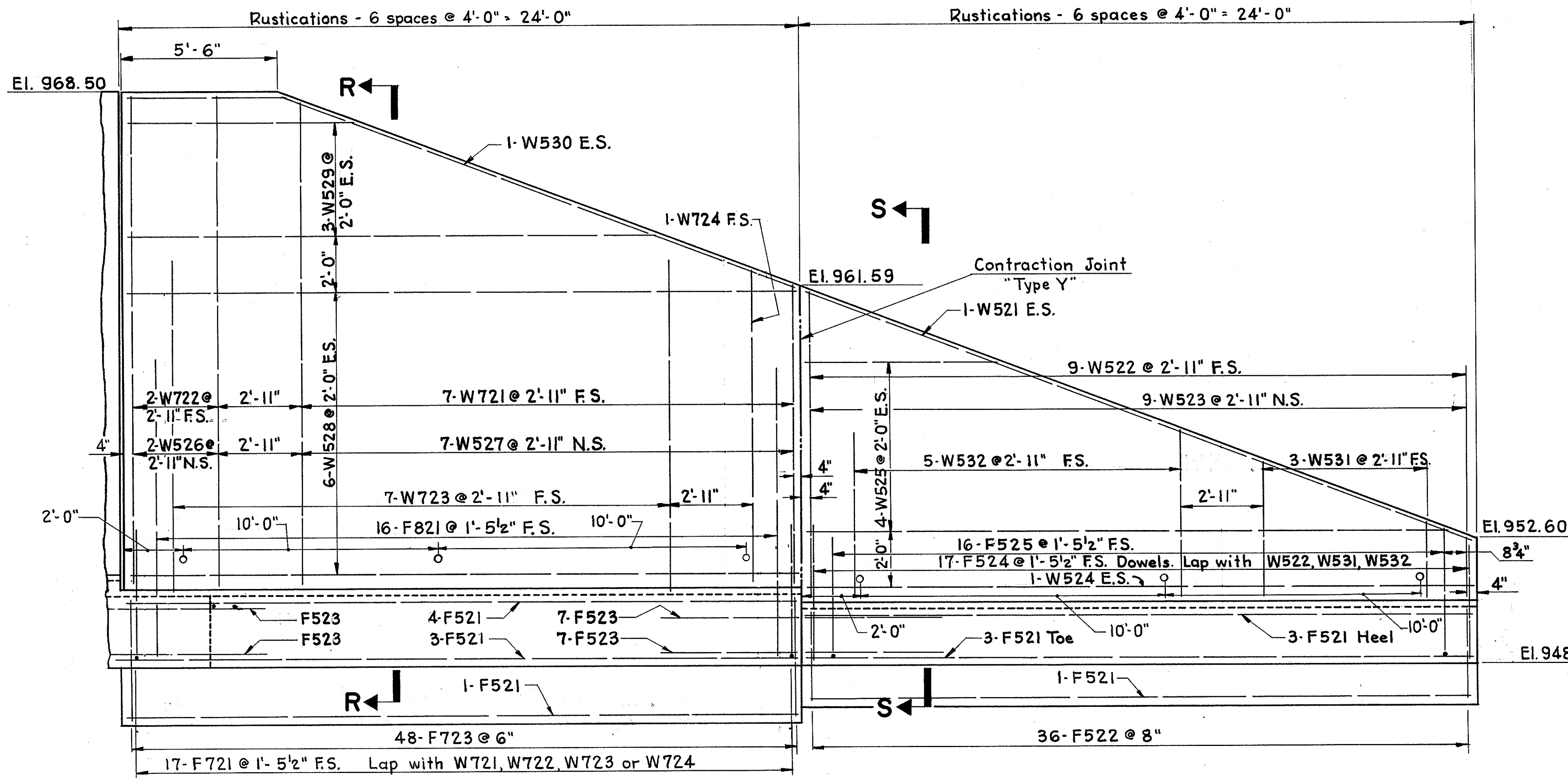


PLAN

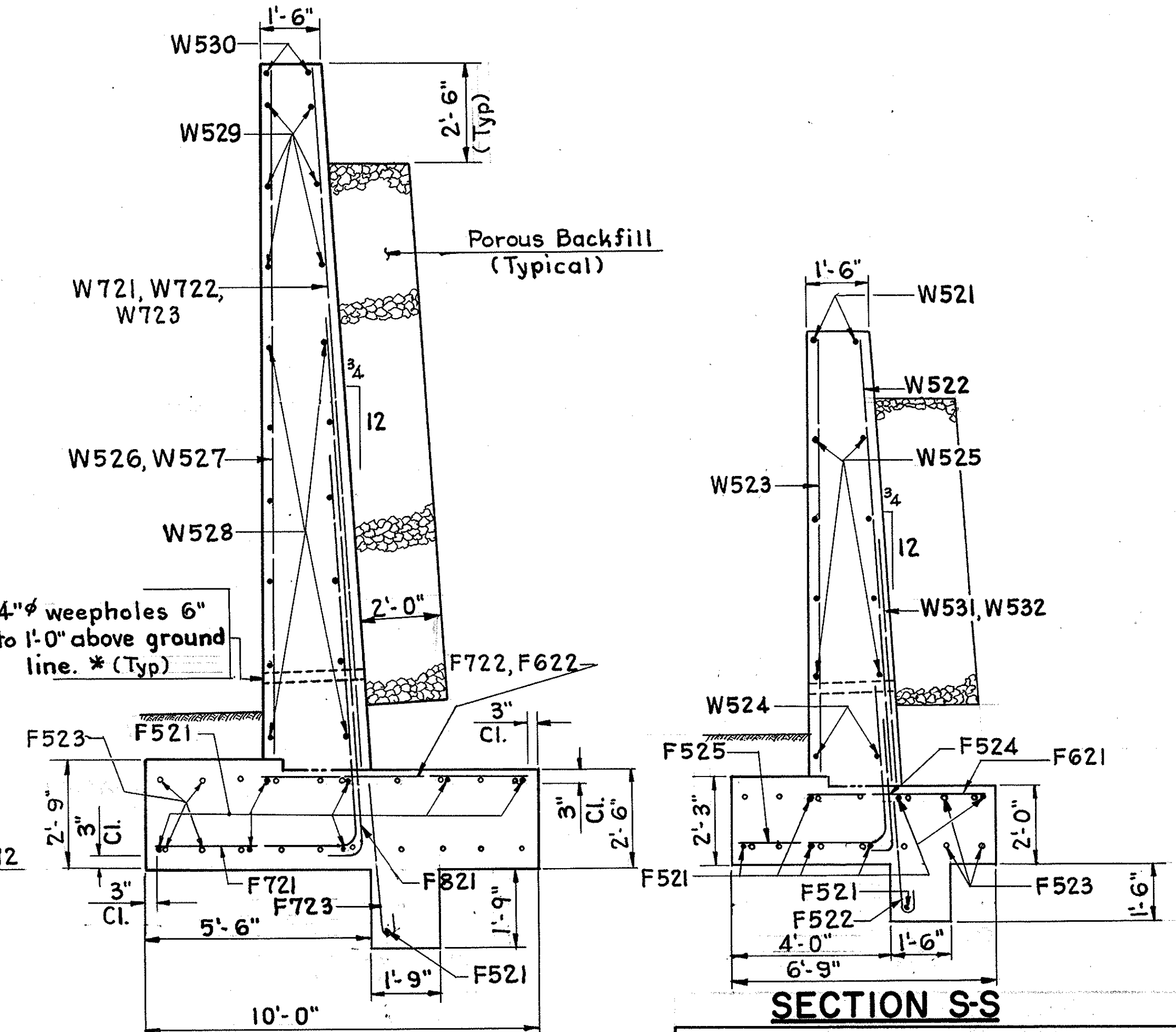


ANCHOR ROD LAYOUT

Abutment B



ELEVATION



SECTION R-R

SECTION S-S

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA
RIGHT RETAINING WALL
ABUTMENT B
BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

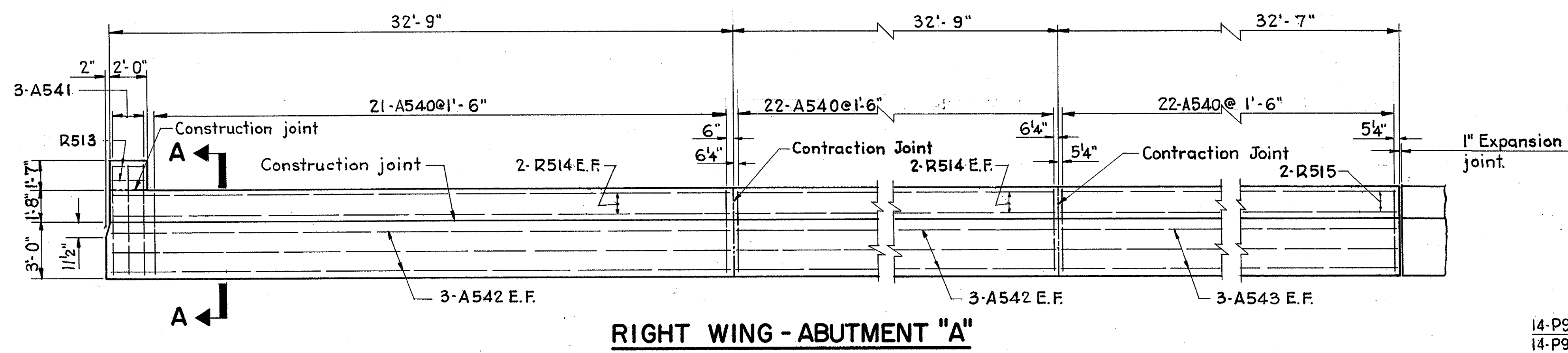
MAHONING CO.		STA. 606+54.21	
Designed	Drawn	Traced	Checked
Reviewed	Date	Revised	
RLS	EFR	P.T.R.	EFR
			FK
			10-14-63

NOTE:
For joint and rustication groove details see Left Retaining Wall Abutment B.

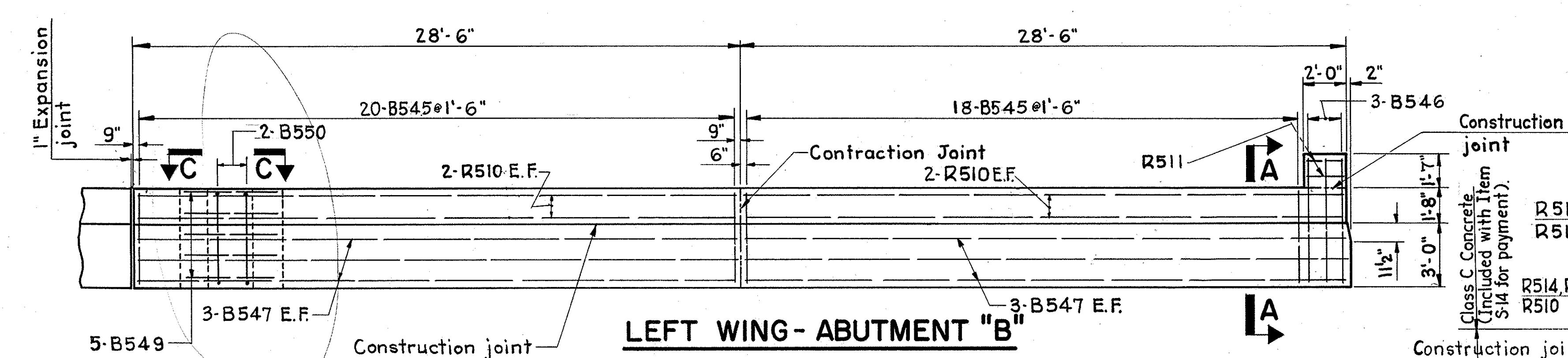
LEGEND
F.S. = Far Side
E.S. = Each Side
N.S. = Near Side

* Locate weepholes as shown on Elevation.

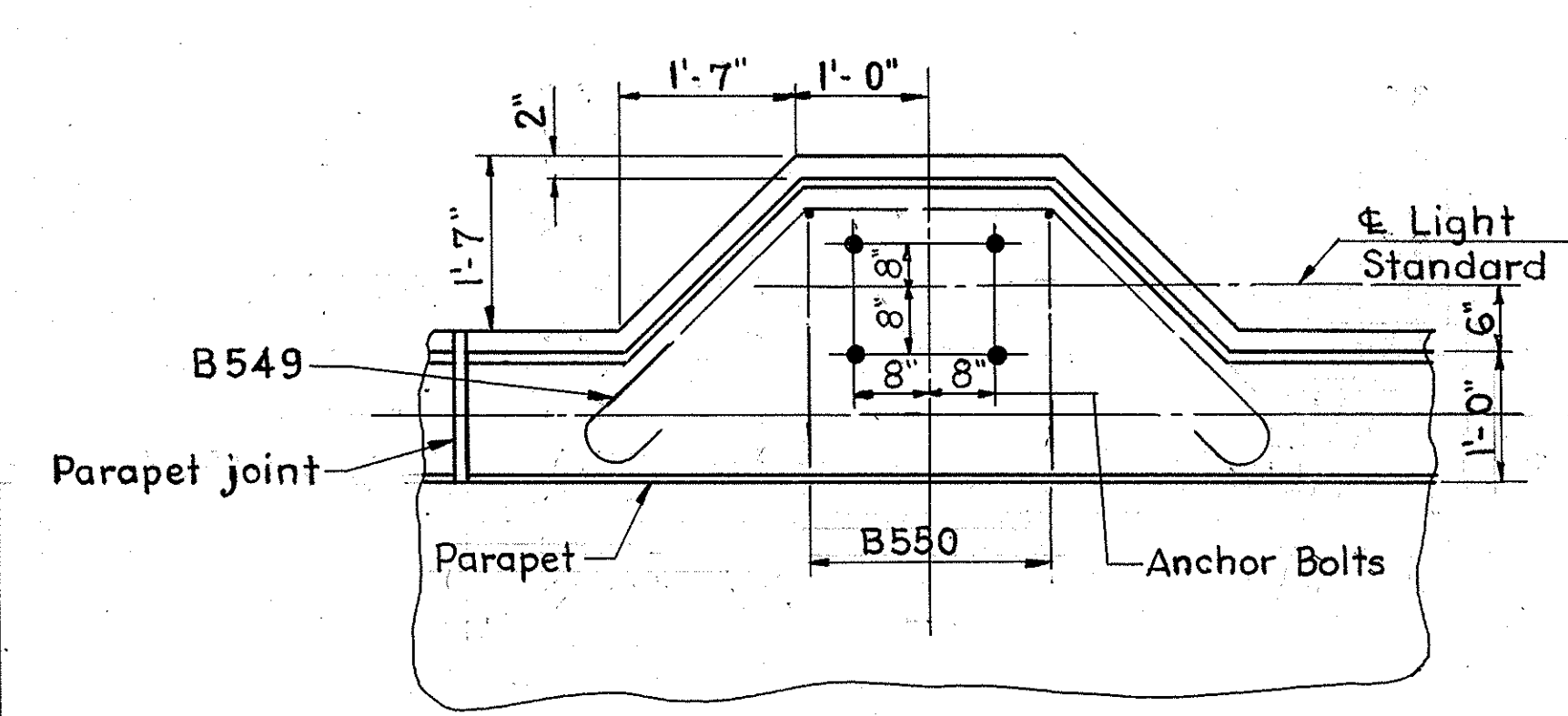
MAHONING COUNTY
MAH-18-15.50



RIGHT WING - ABUTMENT "A"

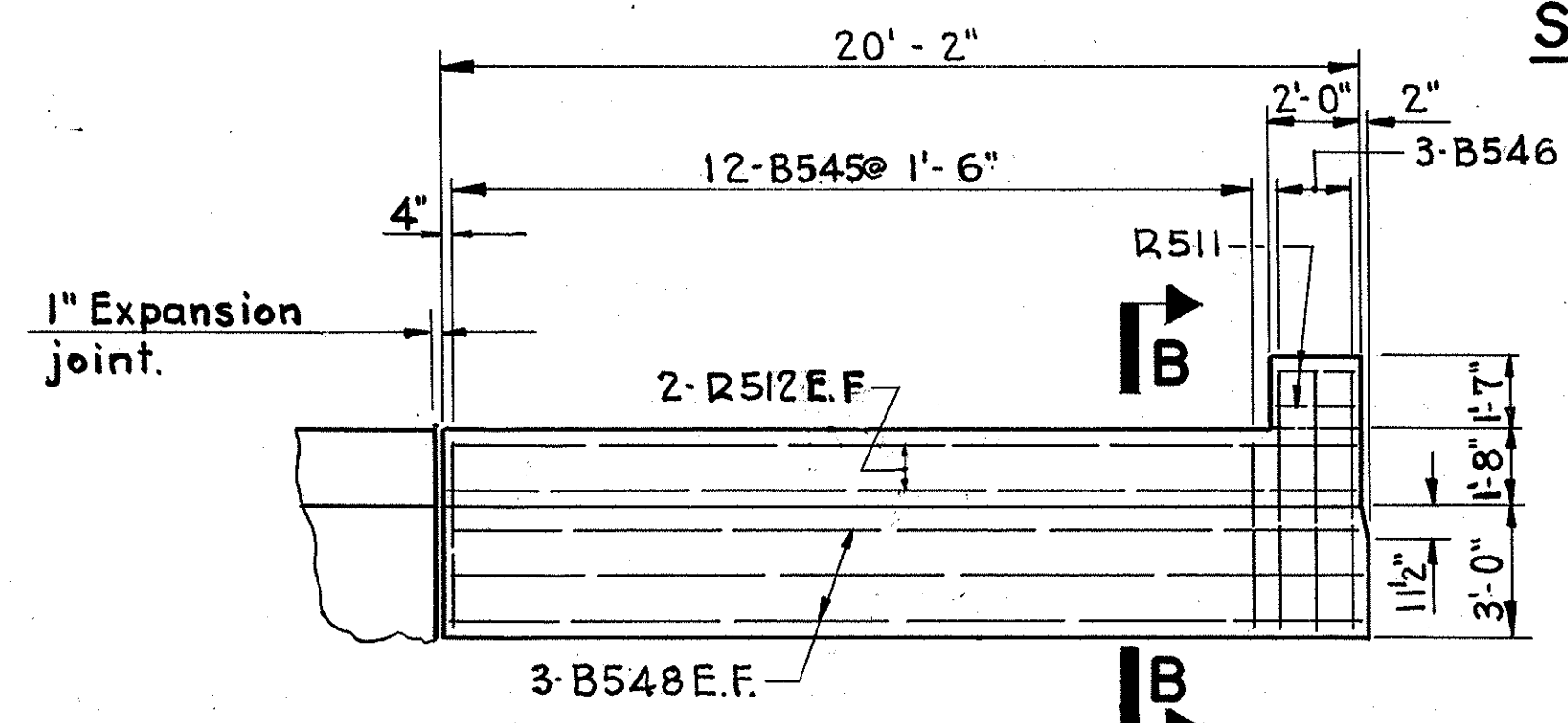


LEFT WING - ABUTMENT "B"

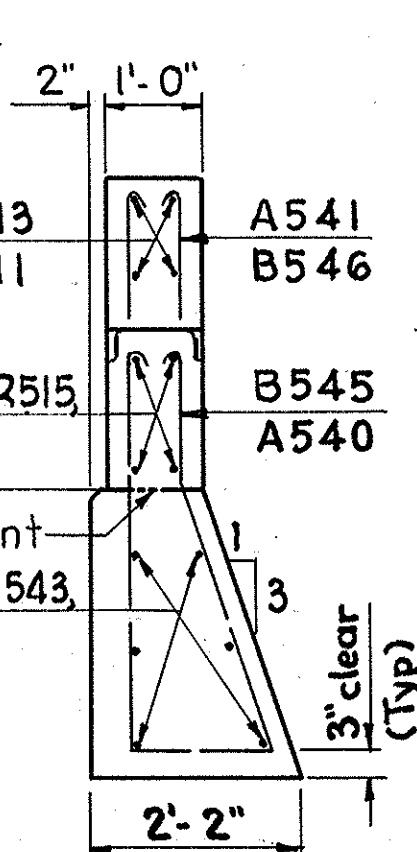


SECTION C-C
PLAN-LIGHT STANDARD BASE

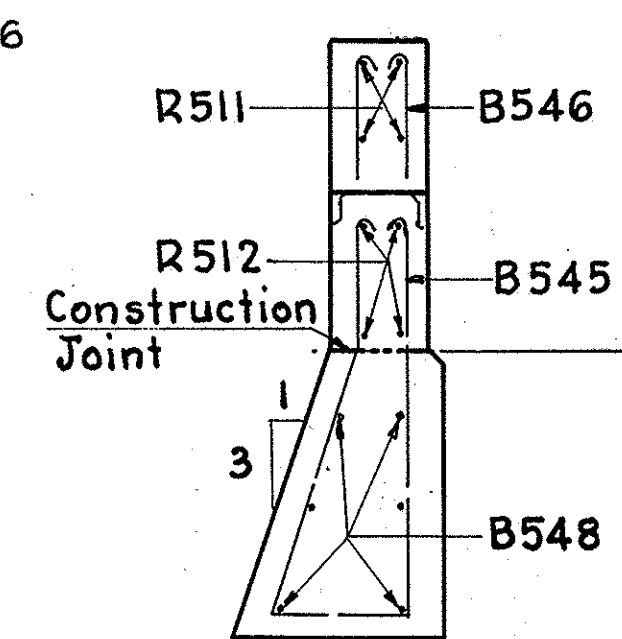
For details not shown see
Details - Bridge Lighting.



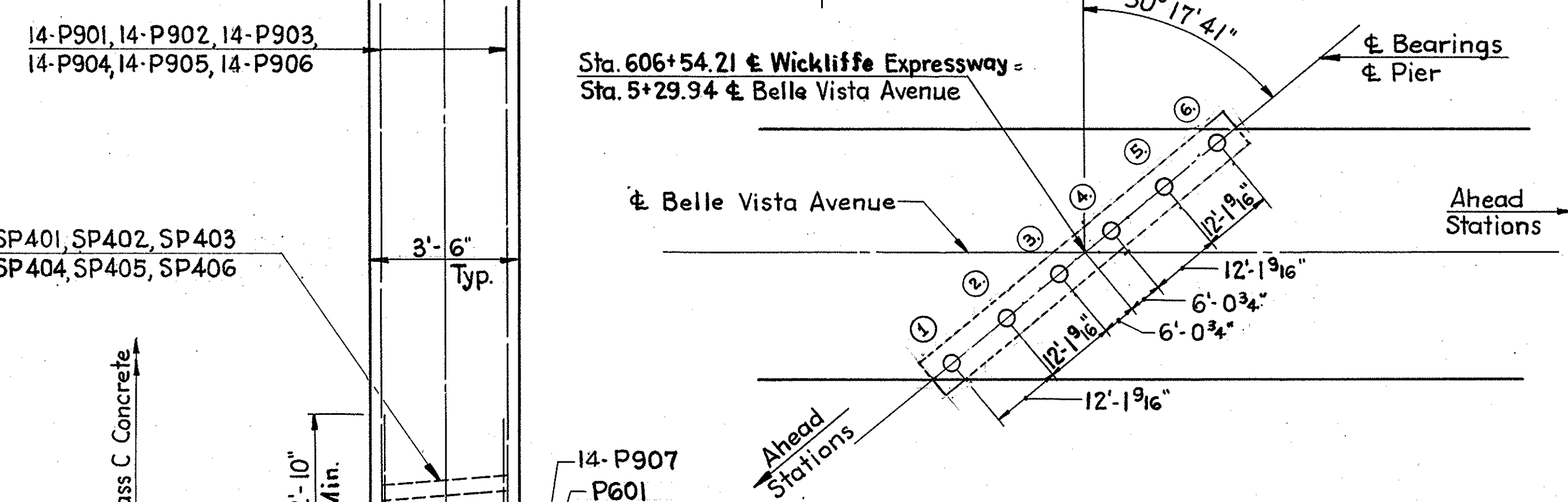
RIGHT WING - ABUTMENT "B"



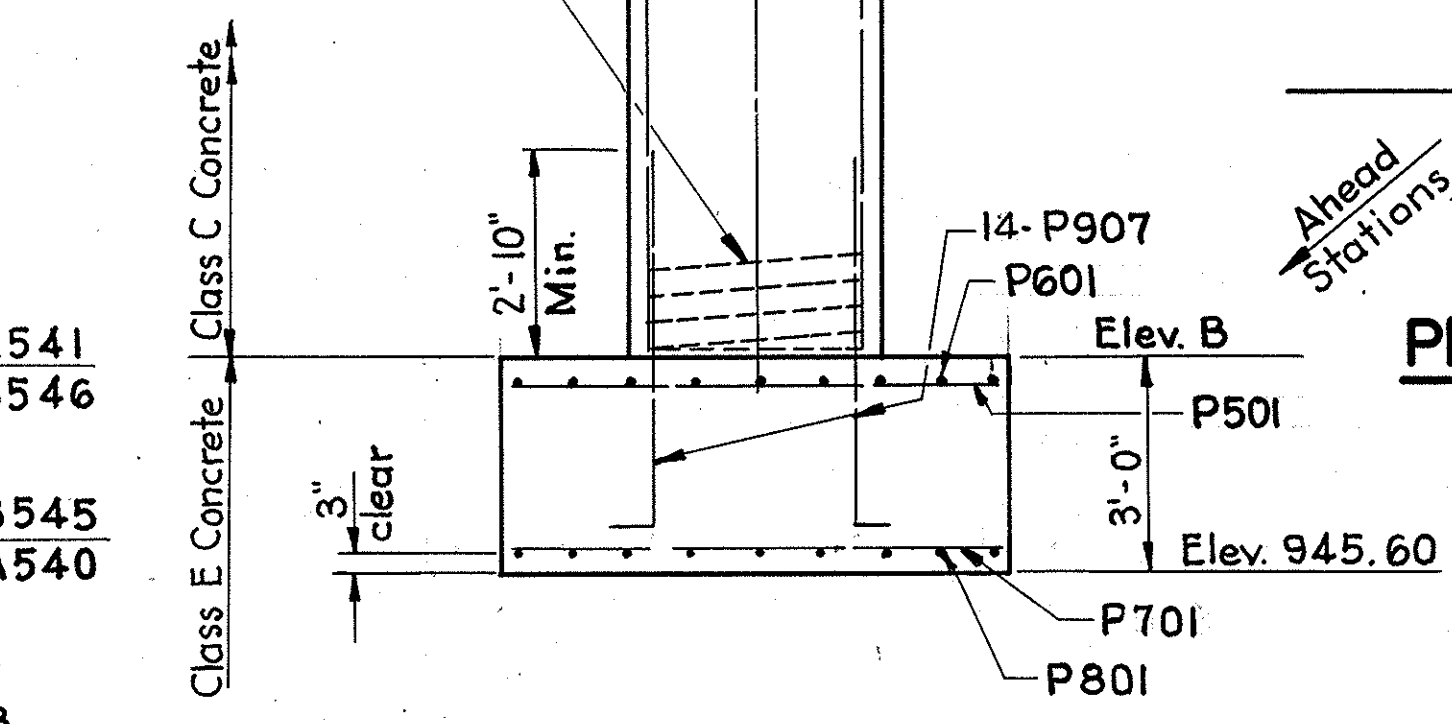
SECTION A-A



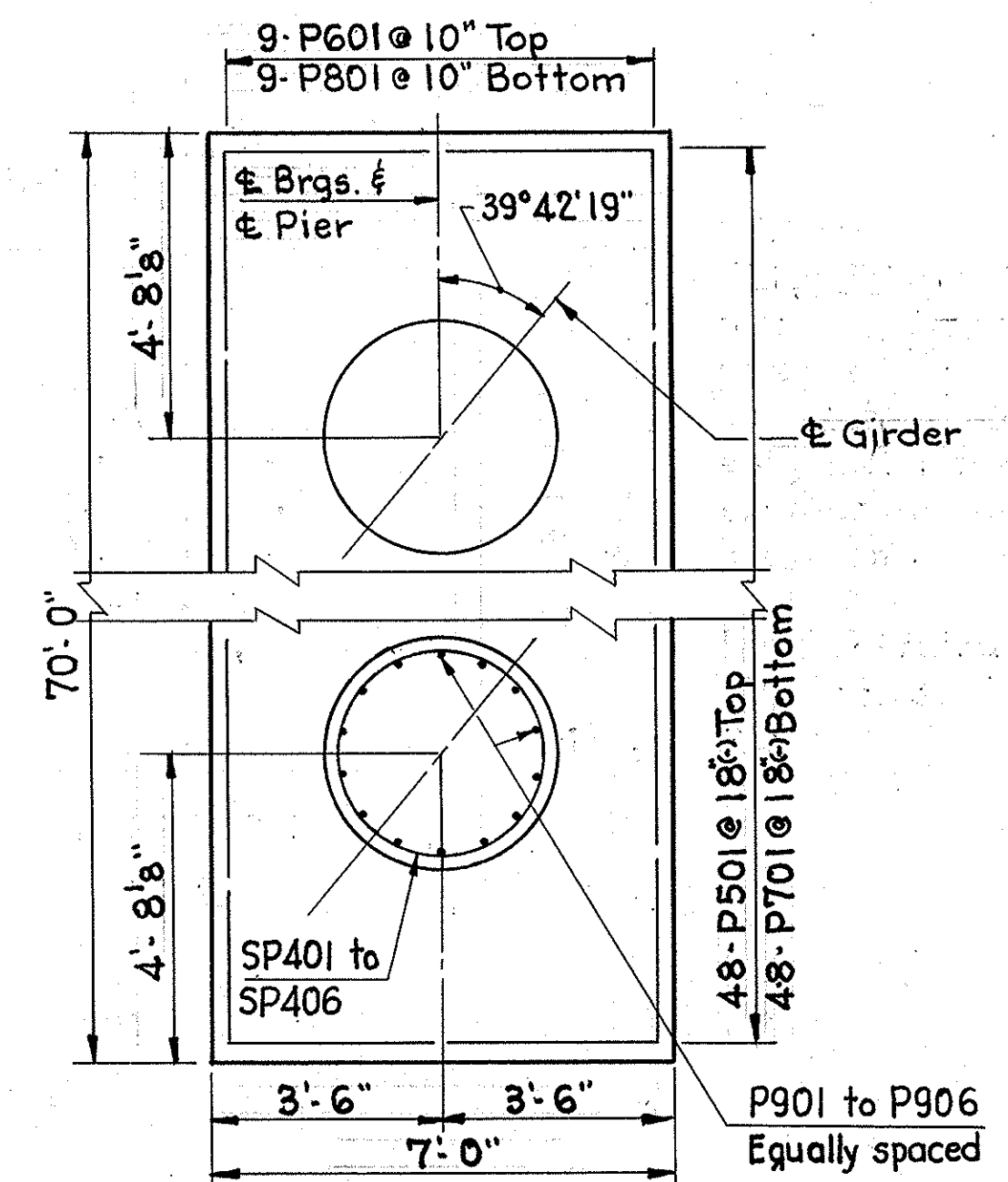
SECTION B-B



PIER LOCATION SKETCH



PIER-ELEVATION



PIER - PLAN

PIER DATA					
Column	Elev. A	Elev. B	Reinforcement		
1	971.19	948.60	P901	SP401	
2	971.03	948.60	P902	SP402	
3	970.86	948.60	P903	SP403	
4	970.56	948.60	P904	SP404	
5	970.13	948.60	P905	SP405	
6	969.69	948.60	P906	SP406	

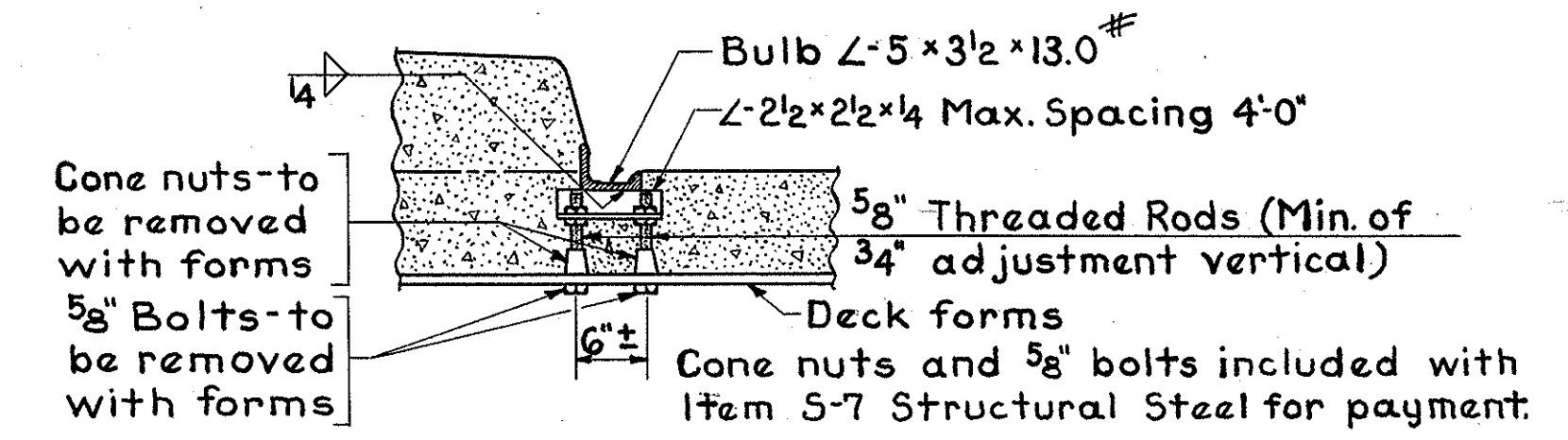
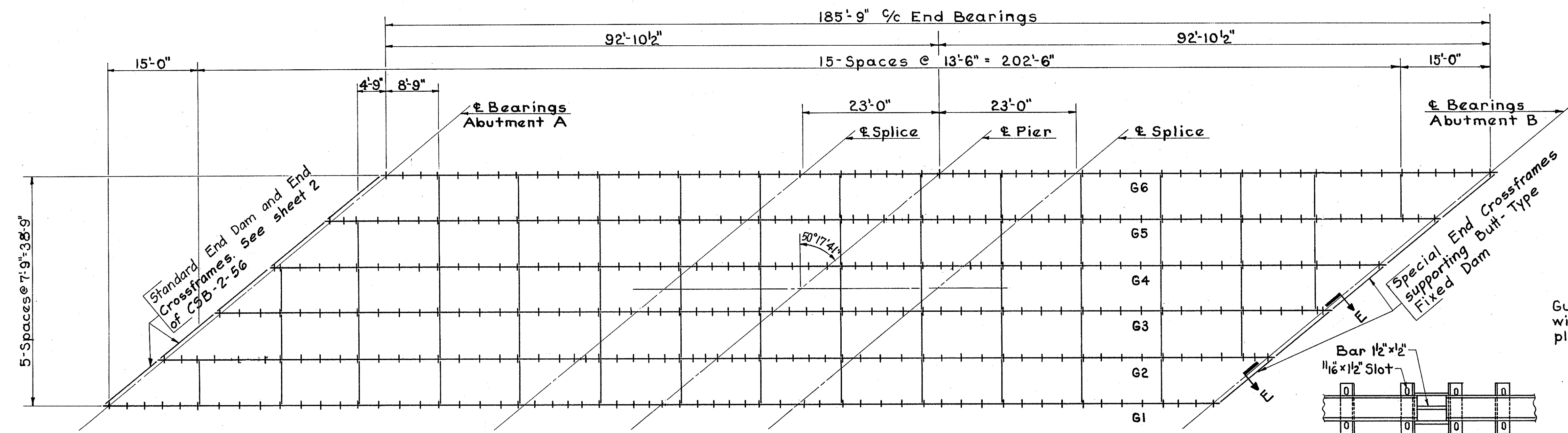
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

PIER & PARAPET WALL
BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

MAHONING CO. STA. 606 + 54.21

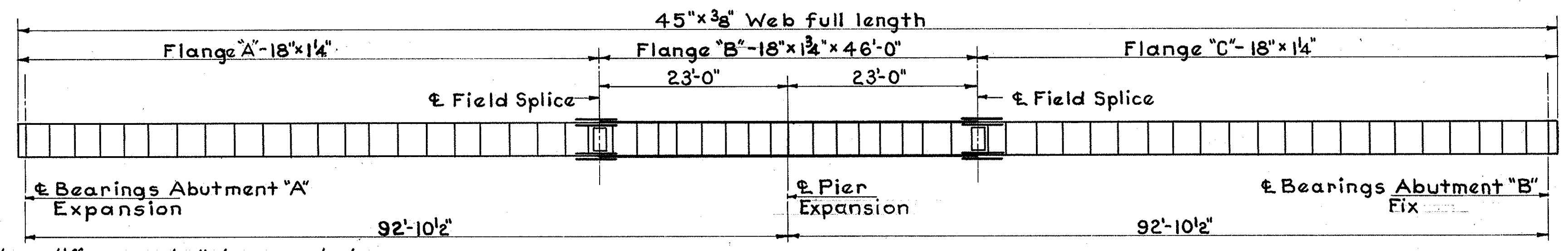
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
RUS	RUS	P.T.R.	EFR	F.K.	10-14-63	

**MAHONING COUNTY
MAH-18-15.50**



Gutters shall be accurately adjusted for alignment and grade, with allowance for dead load deflection before concrete is placed. Gutters shall not be used to support finishing machines.

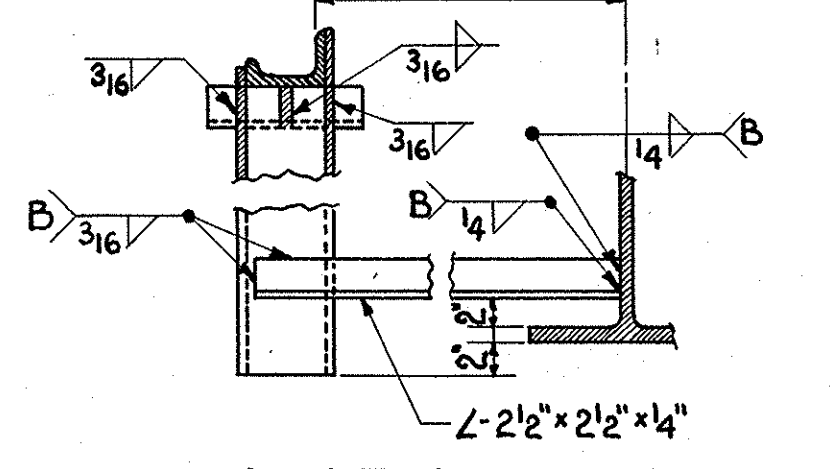
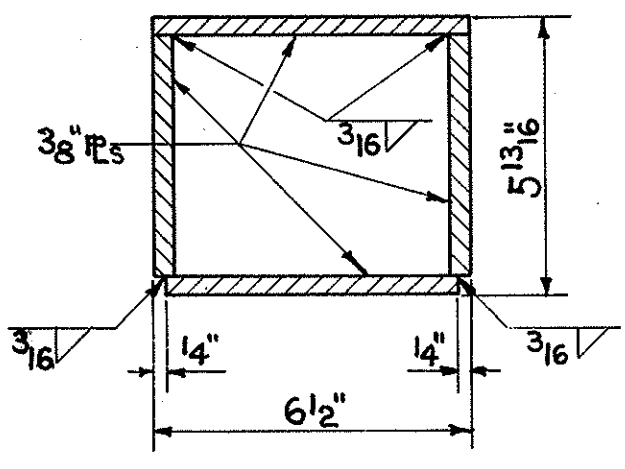
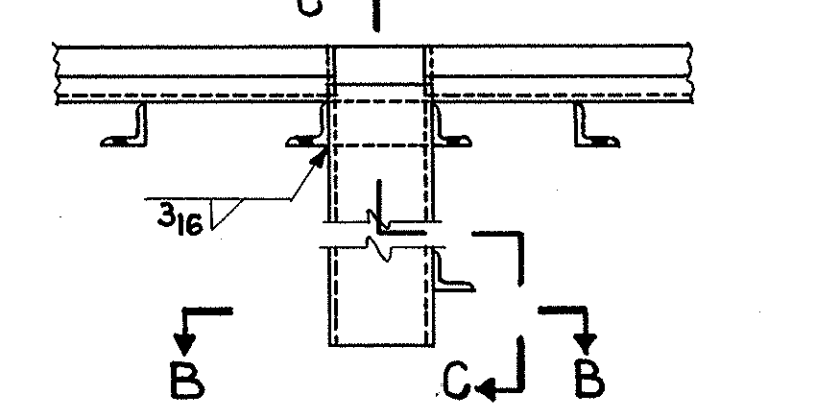
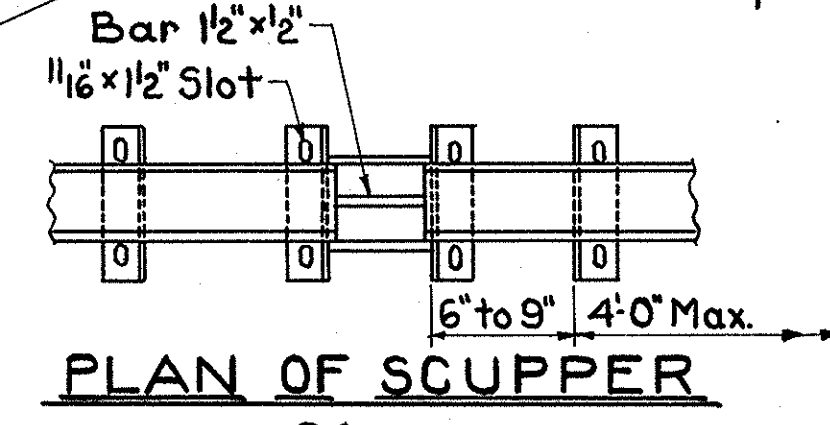
STEEL FRAMING PLAN



Intermediate stiffeners shall have contact bearing with the compression flange, but may have a clearance of not more than 1/8 inch from the tension flange. In shop painting, care shall be taken to make certain that paint is forced through from one side to the other of the 5/8 inch opening.

GIRDER ELEVATION (G1 Shown)

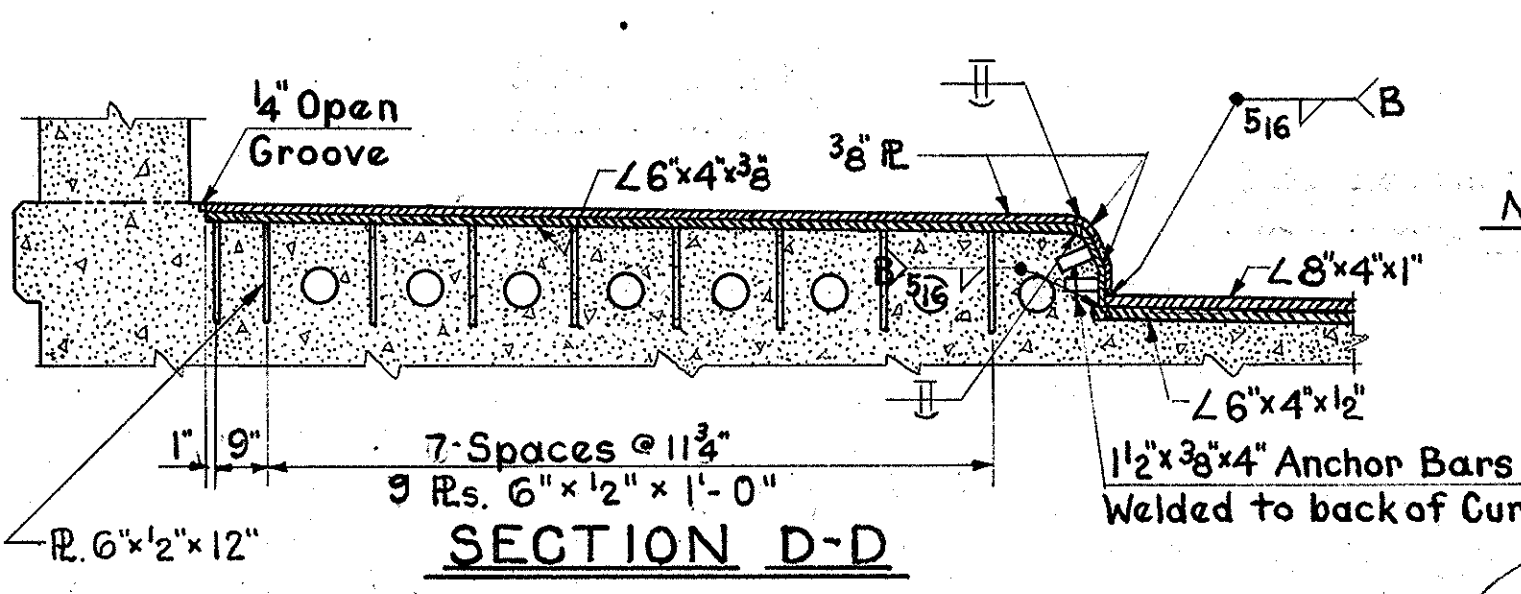
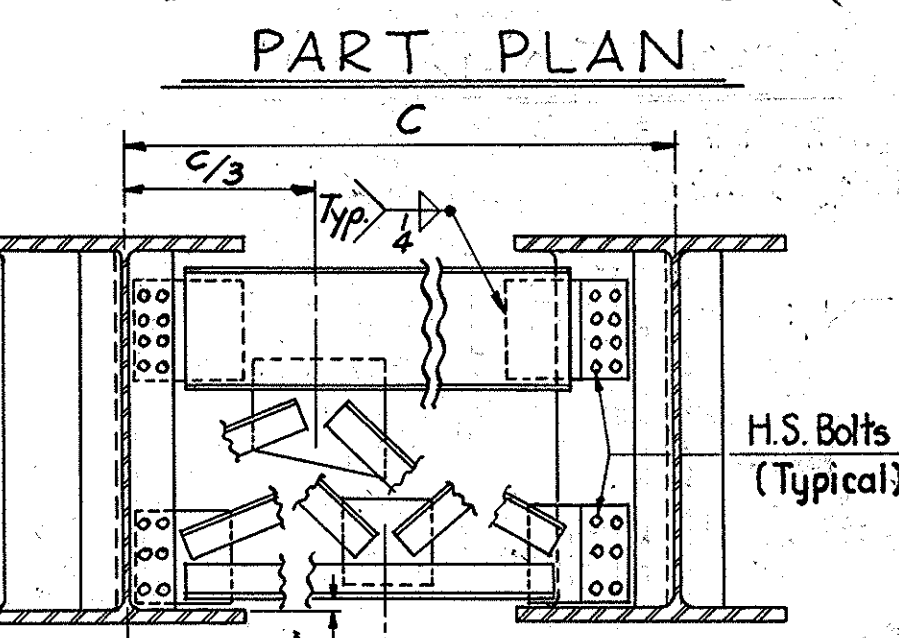
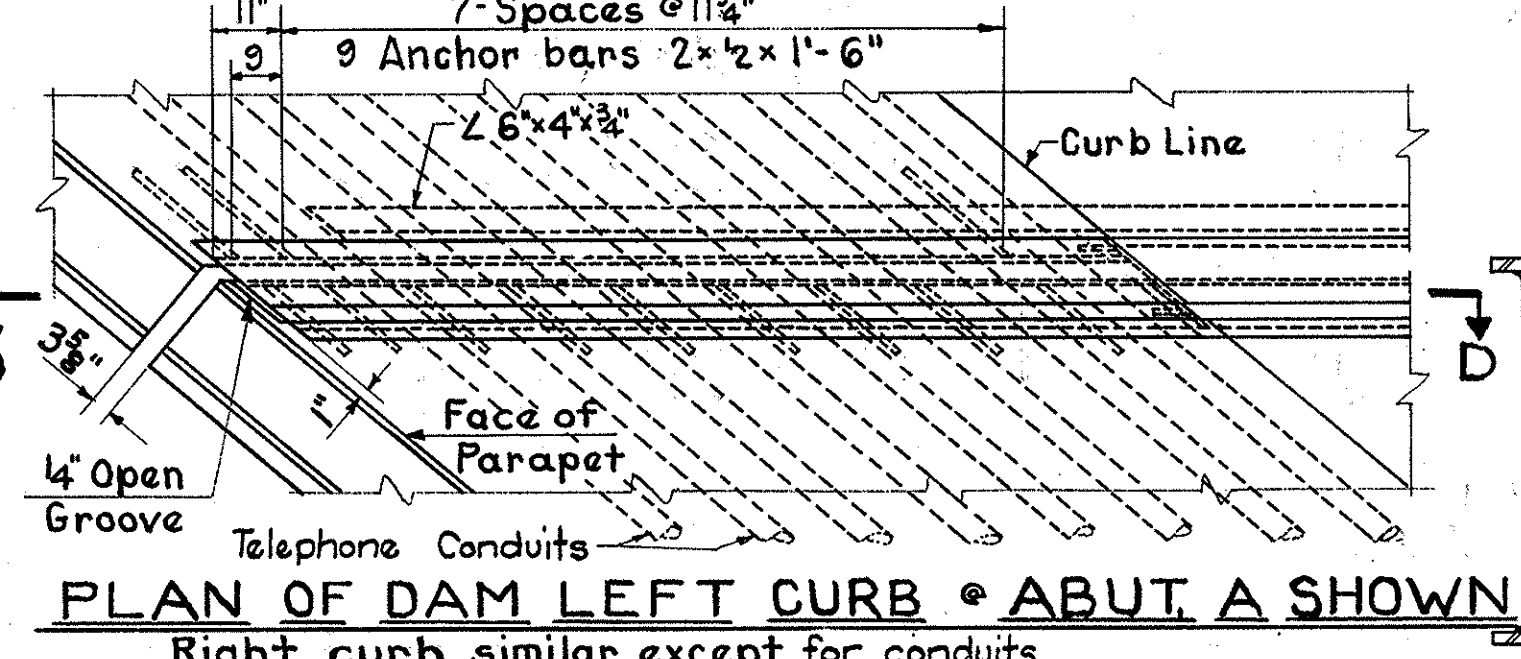
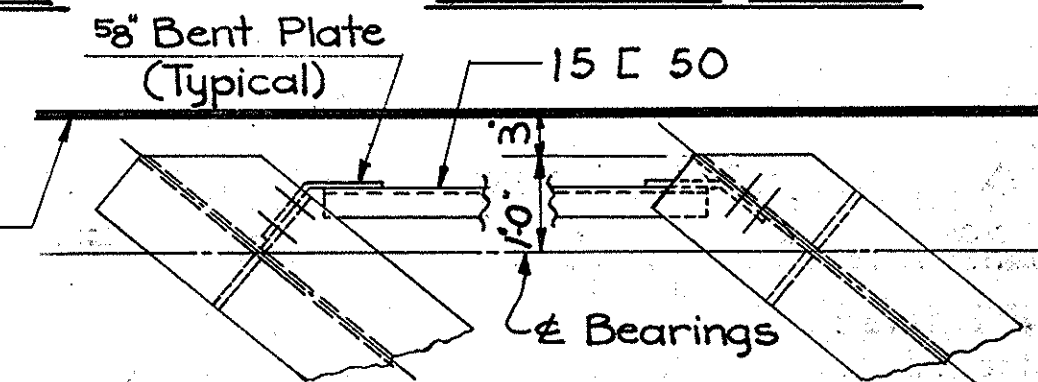
Note: For electrical ground details, see General Notes on General Shop Splices: If additional shop splices are necessary, their location and detail shall be submitted to the Director for approval prior to ordering of material.



ELEVATION OF SCUPPER

SECTION B-B

SECTION C-C



NOTE: See sheet 2 of CSB-2-56 for size of angles and plates and for welding details.

See sheet 271 for Deflection and Camber data.

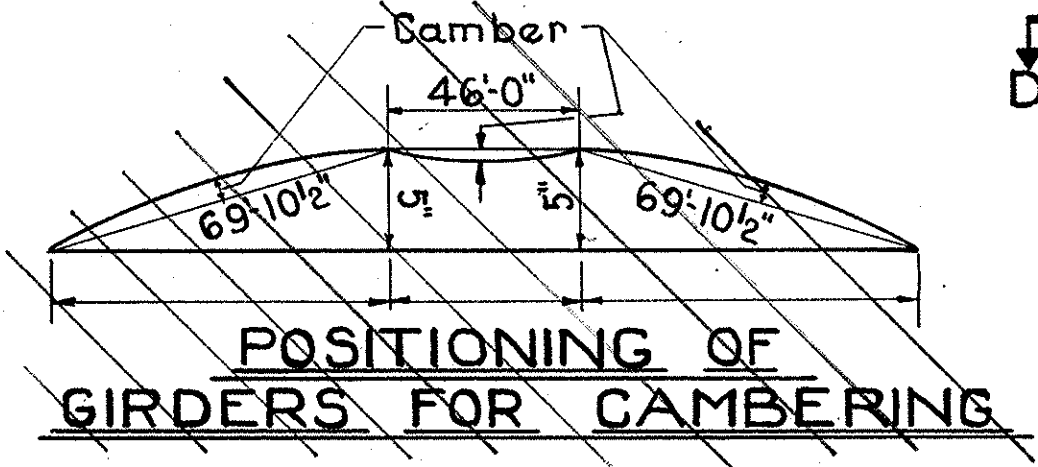
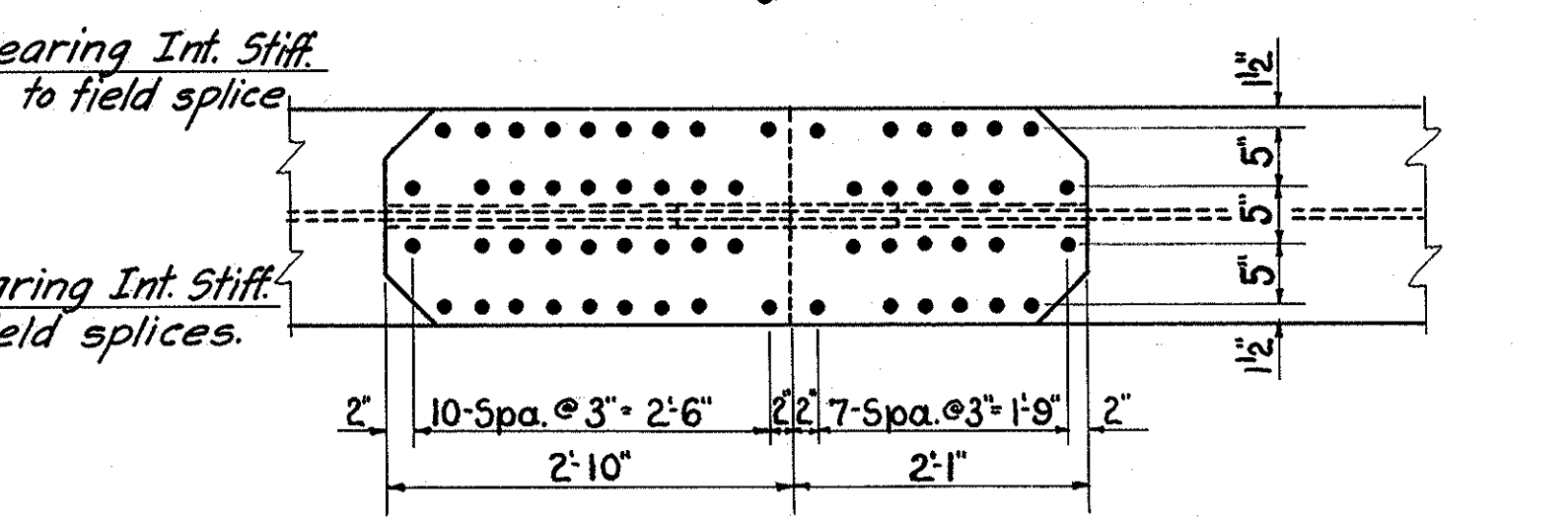
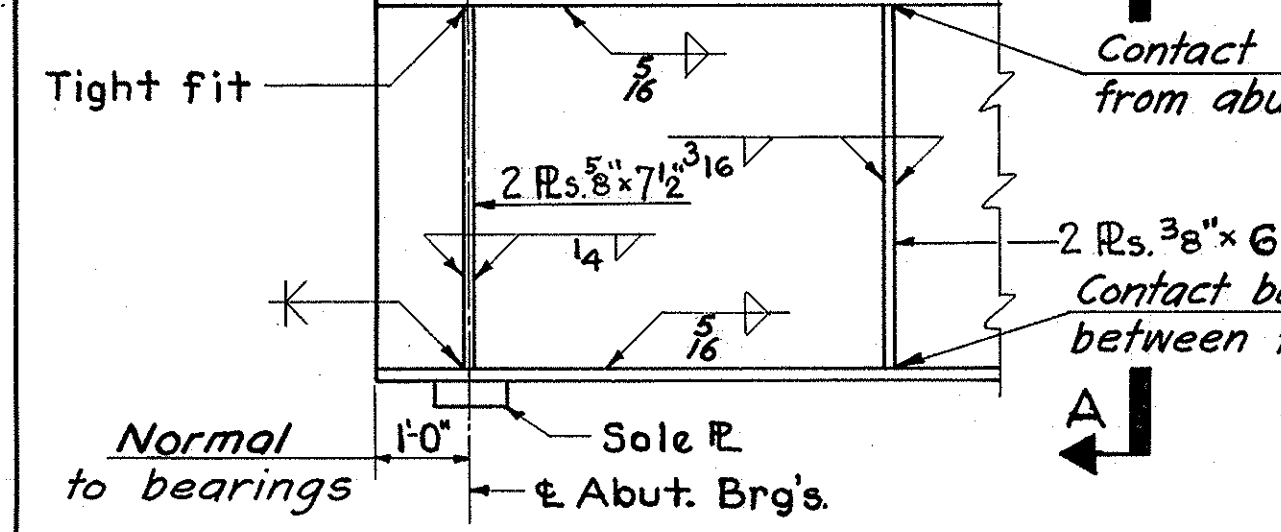
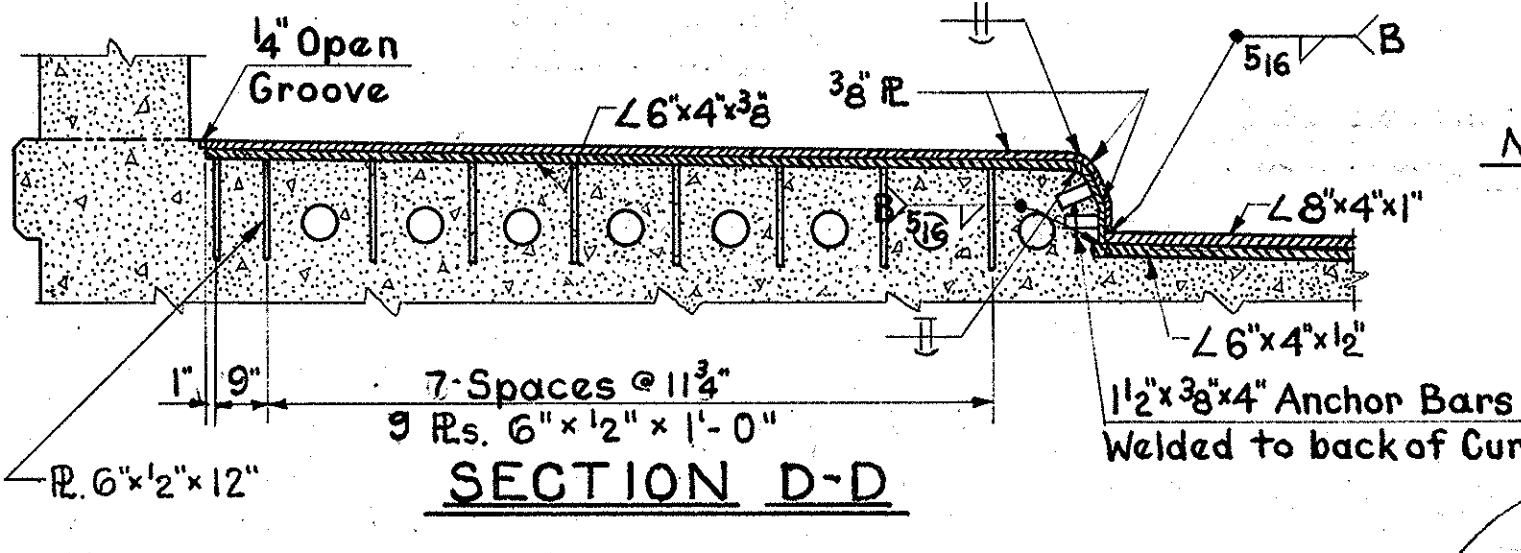


TABLE OF GIRDER MATERIALS & BEARINGS

Web	45" x 3/8"
Flange "A"	18" x 1 1/4"
Flange "B"	18" x 1 1/4"
Flange "C"	18" x 1 1/4"
Abutment Bearing Stiffeners	2(7 1/2" x 5/8")
Pier Bearing Stiffeners	2(7 1/2" x 3/8")
Intermediate Stiffeners	2(6" x 3/8")
Abutment "A" Bearings	E-150
Abutment "B" Bearings	F-150
Pier Bearings	R-250



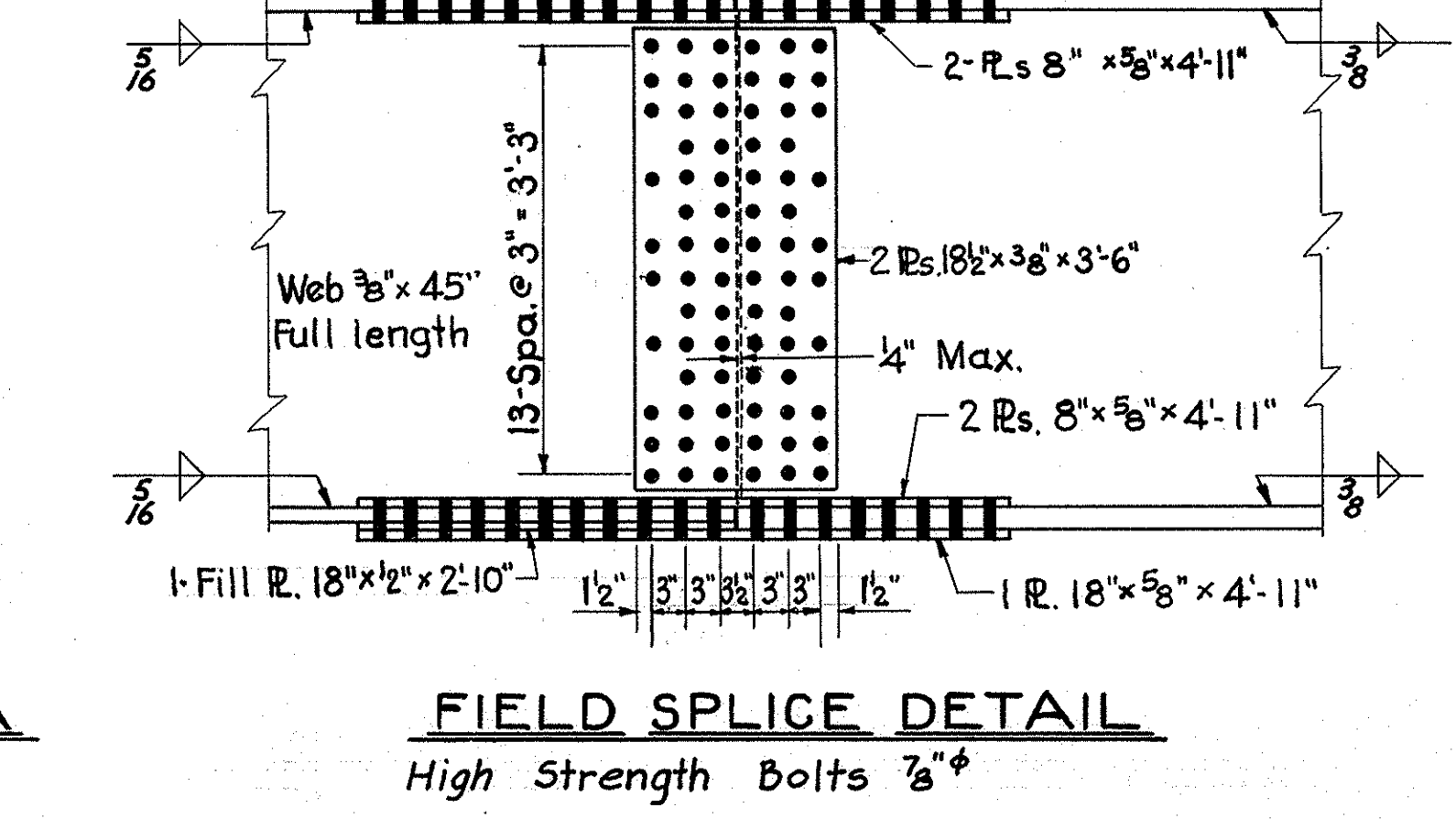
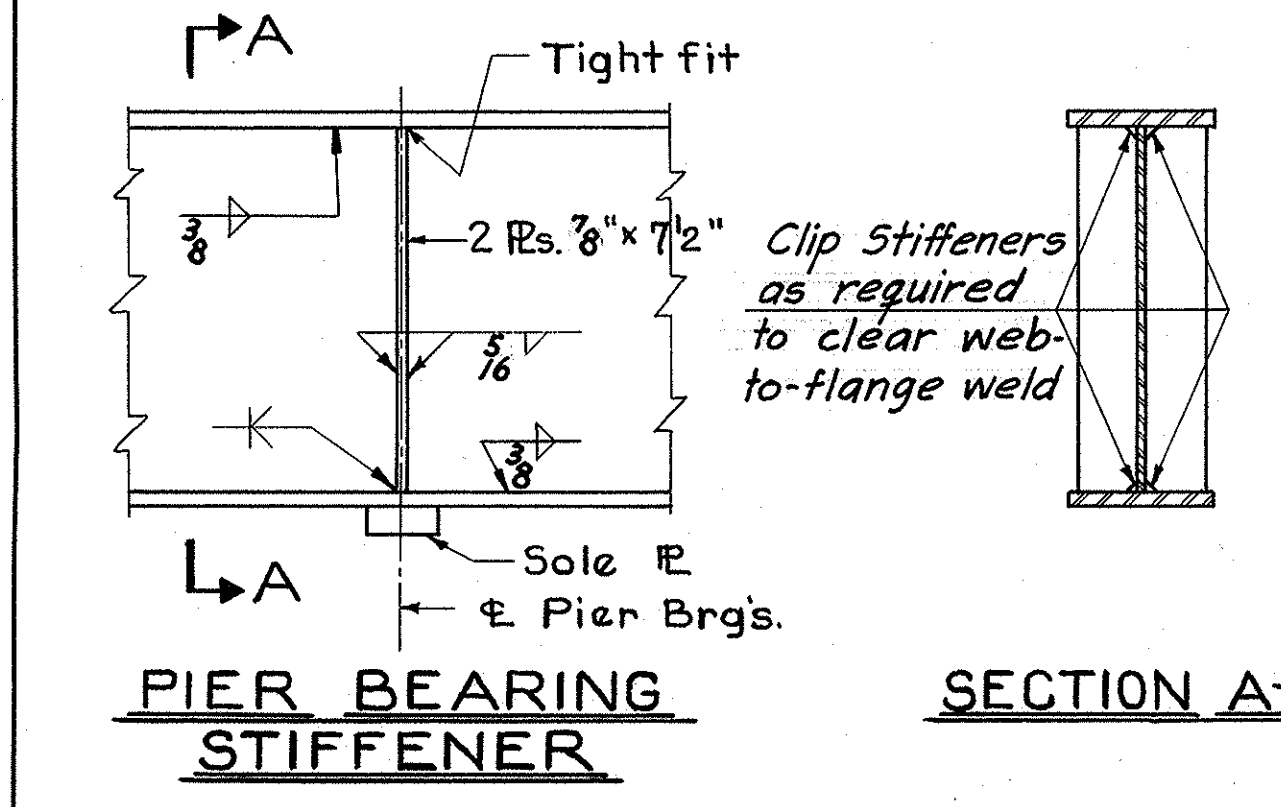
Note: For Notes see other Superstructure Details sheet.

ABUTMENT BEARING STIFFENER & INTERMEDIATE STIFFENER

FIELD SPLICE DETAIL

DEFLECTION & CAMBER

LOCATION	SECTION A		SECTION B				SECTION C											
	G1	G2	G3	G4	G5	G6	G1	G2	G3	G4	G5	G6						
Deflection due to Steel	3 1/8"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	3 1/8"	-8"	-	-	-	-8"	3 1/8"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	1 1/4"	3 1/8"
Deflection due to Remaining Dead Load	1 1/8"	3/4"	3/4"	3/4"	3/4"	1 1/8"	-	-	-	-	-8"	1 1/8"	3/4"	3/4"	3/4"	3/4"	3/4"	1 1/8"
Convexity Required for Vertical Curve	3/4"	3/4"	3/4"	3/4"	3/4"	3/4"	-	-	-	-	-3/8"	1/2"	3/8"	1/4"	1/8"	0"	0"	3/4"
Sum of Deflection and Convexity	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	-	-	-	-	-3/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"
Required Camber	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	-	-	-	-	-8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"	1 1/8"



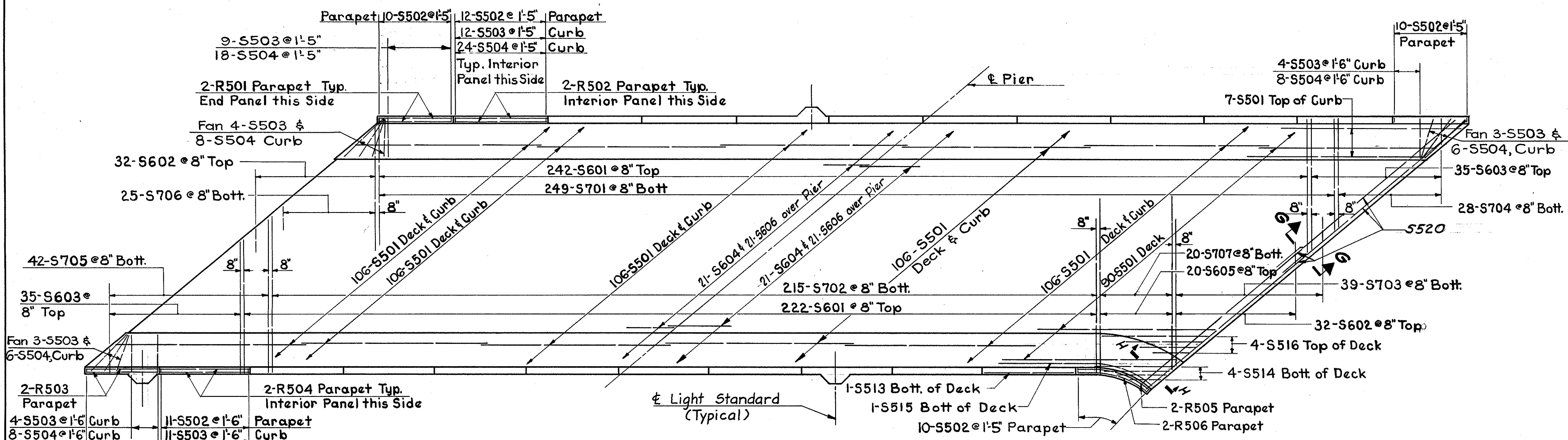
MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS

BRIDGE NO. MAH-18-1715
UNDER BELLE VISTA AVENUE

MAHONING CO.	STA 606+54.21					
Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
RLS	J.H.	J.H.	EFR	FK	10-14-63	2-23-66

MAHONING COUNTY
MAH-18-15.50



DECK REINFORCING PLAN

Notes:
For Reinforcing not shown see Detail "A"
See General Plan for location of curb and parapet radii.

NOTES:

- Refer to *Std. Dwg. CSB-2-56, revised 2-2-59, sheets 2 & 3 of 6* for details of End Dams, End Crossframes, 2" Pipe Drains, Curb Plates and Bulb Angles and to Standard Drawing AR-1-57 for details of Type "C" Aluminum Railing and Concrete Parapet and to Standard Drawing FS B-1-62 for details of Fixed and Expansion bearings, also *RB-1-55, revised 2-2-59*.
- ERECTION PROCEDURE: The contractor shall submit 3 prints, showing the proposed Structural Steel erection procedure to the Director for approval.
- CONCRETE shall be class "C" Concrete and reinforcing steel above parapet construction joint shall be included with railing for payment.
- See General Plan and Elevation for location of scuppers, 2" pipe drains and railing posts.
- CONCRETE DECK PLACING: In order to facilitate water curing of the concrete of the deck slab, the placing of concrete shall progress upgrade. The slab may be placed in sections, between transverse construction joints which are parallel to transverse reinforcing steel and are located near the center of any span.
- DECK SLAB HAUNCH: The haunch in the deck slab adjacent to the top of steel beams which is shown as 9" wide, may vary from this dimension between the limits of 6" and 12" except that the maximum slope shall not exceed 3 inches per foot. Payment for deck slab concrete shall be based on the 9" width.
- MACHINE FINISH: The concrete bridge deck shall be finished by the use of a finishing machine.
- ELECTRICAL LIGHTING SYSTEM: For additional details, refer to drawings, "Details-Bridge Lighting" and "General Lighting Notes."

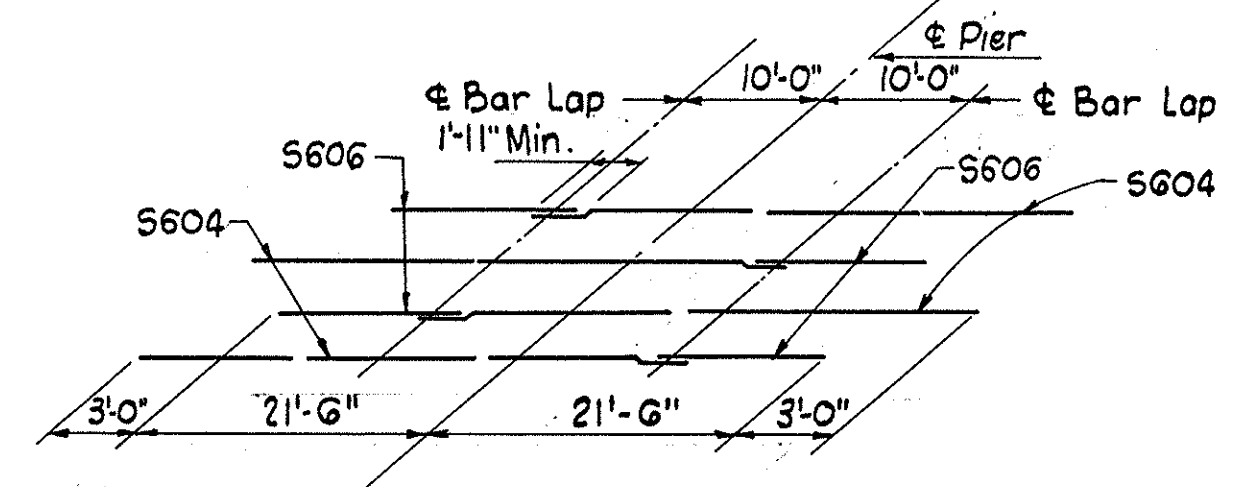
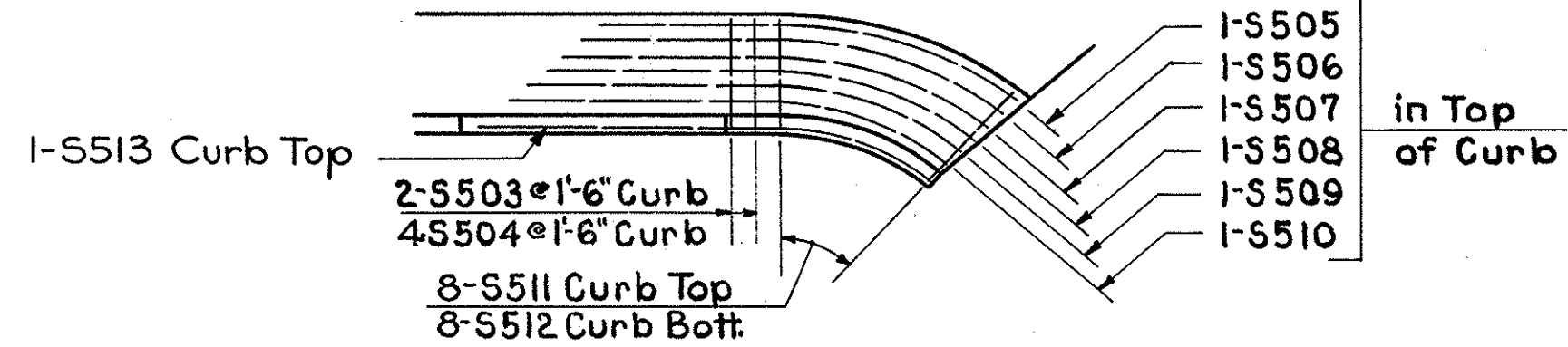
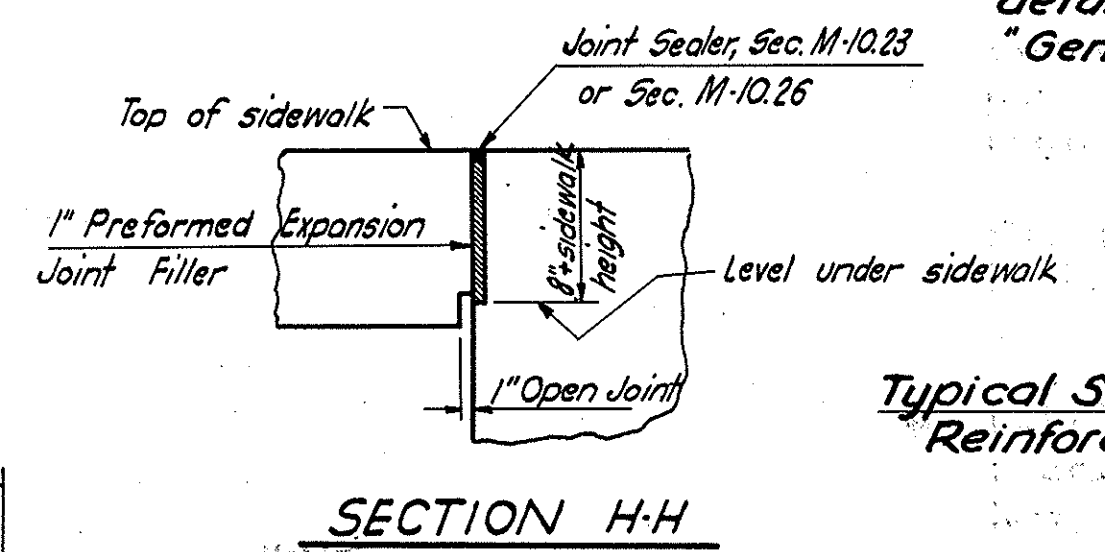


DIAGRAM SHOWING STAGGER OF S604 & S606 BARS

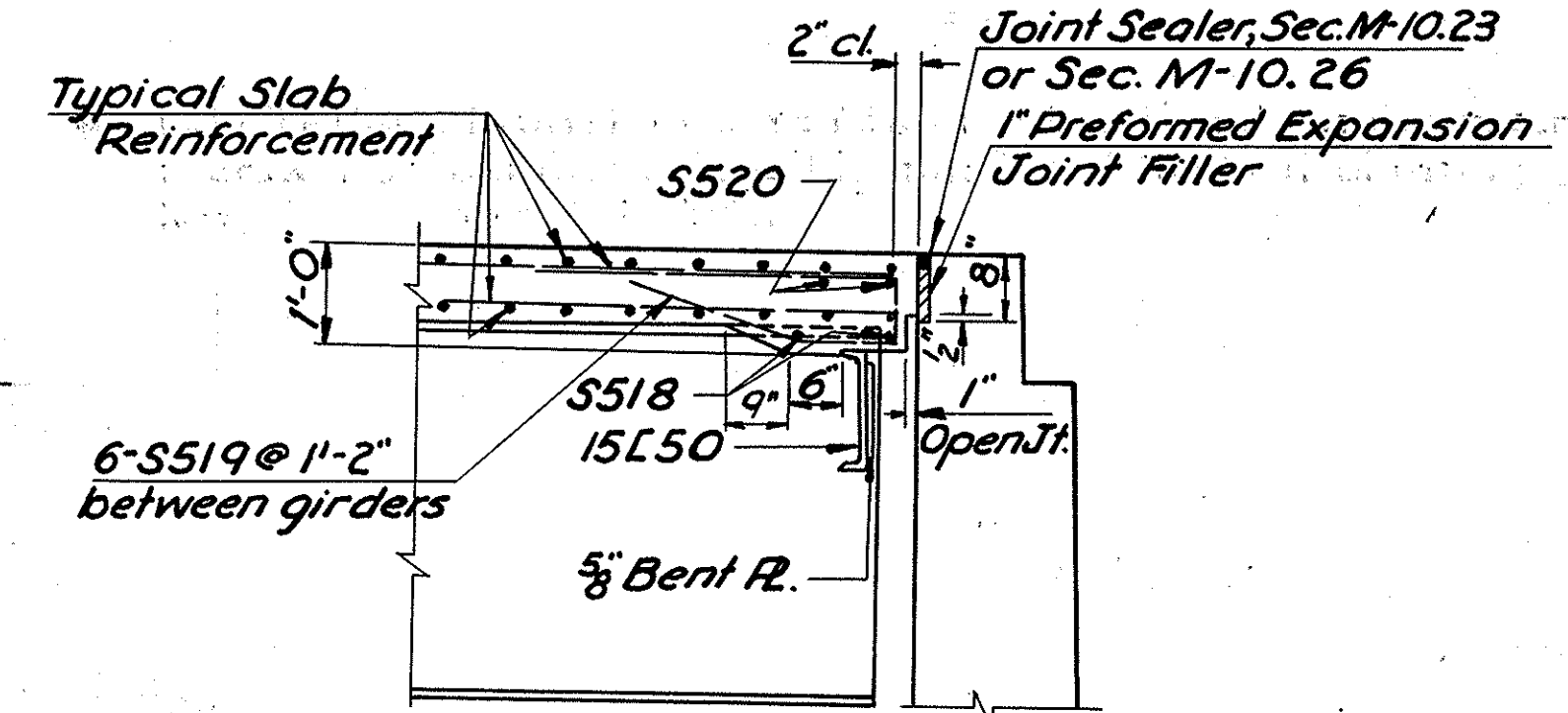
* This is the nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation necessary to place it parallel to the finished grade.



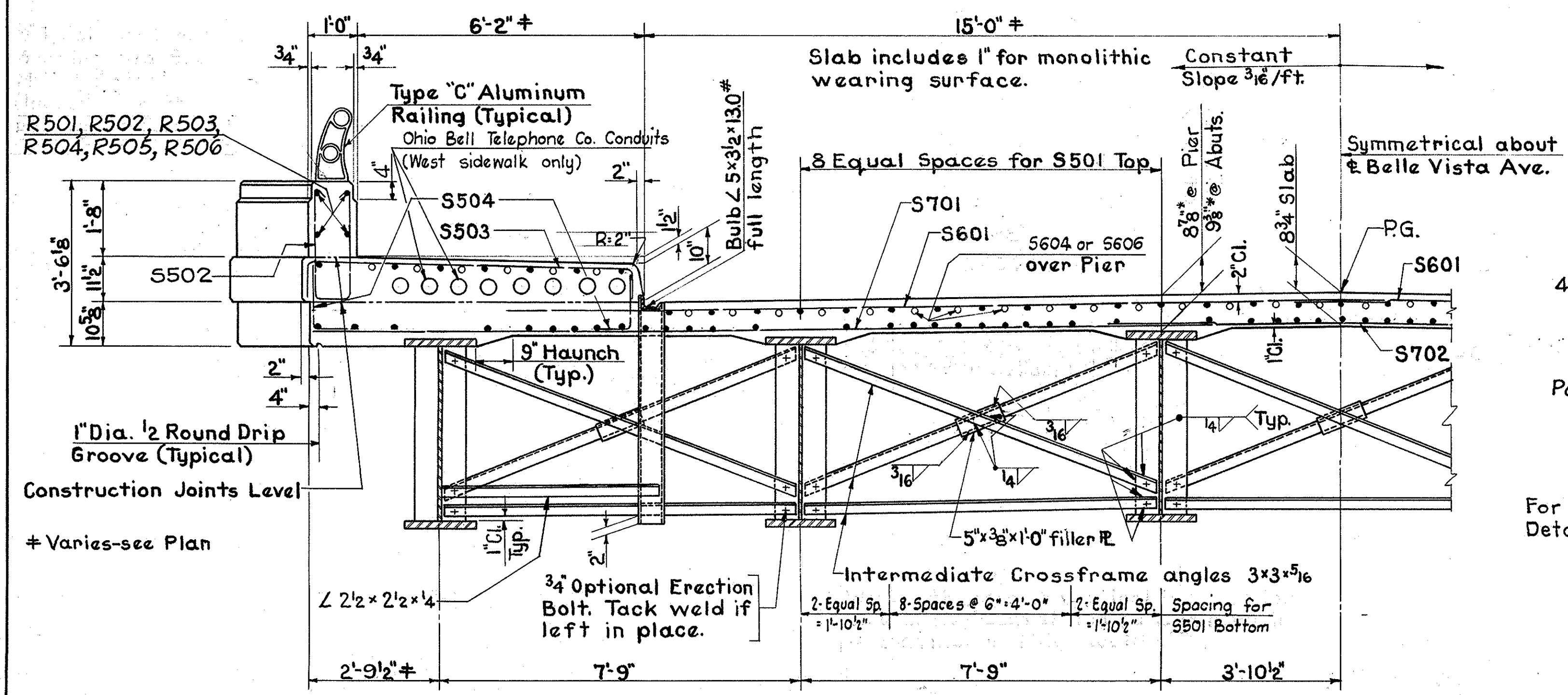
DETAIL "A"



SECTION H-H



SECTION G-G



HALF CROSS SECTION

For details not shown see Details - Bridge Lighting.

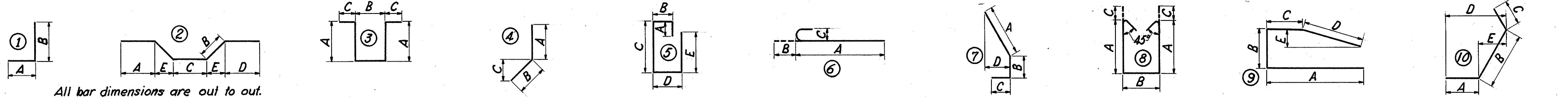
DETAIL B
PLAN-LIGHT STANDARD BASE

For location see General Plan & Elevation

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
SUPERSTRUCTURE DETAILS					
BRIDGE NO. MAH-18-1715					
UNDER BELLE VISTA AVENUE					
MAHONING COUNTY			STA. 606 + 54.21		
Designed	Drawn	Traced	Checked	Reviewed	Date
RLS	J.H.	CJB	EFR	FK	10-14-63

MAHONING COUNTY MAH-18-15.50

* 2 Ea. vary A by 2'-0"

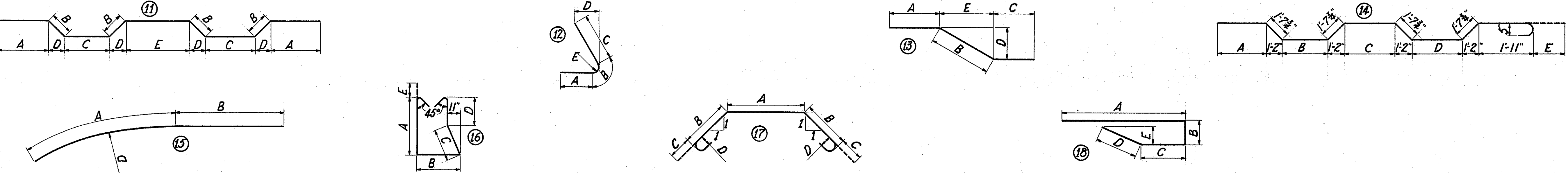


All bar dimensions are out to out.

Table with columns: Mark, Total, Length, Type, A, B, C, D, E, Weight. Contains data for ABUTMENT A, ABUTMENT A (CONTINUED), ABUTMENT B, and ABUTMENT B (CONTINUED). Includes a 'Total' row at the bottom of the table with a value of 20,777.

Included with railing for payment

Included with Railing for payment



MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA

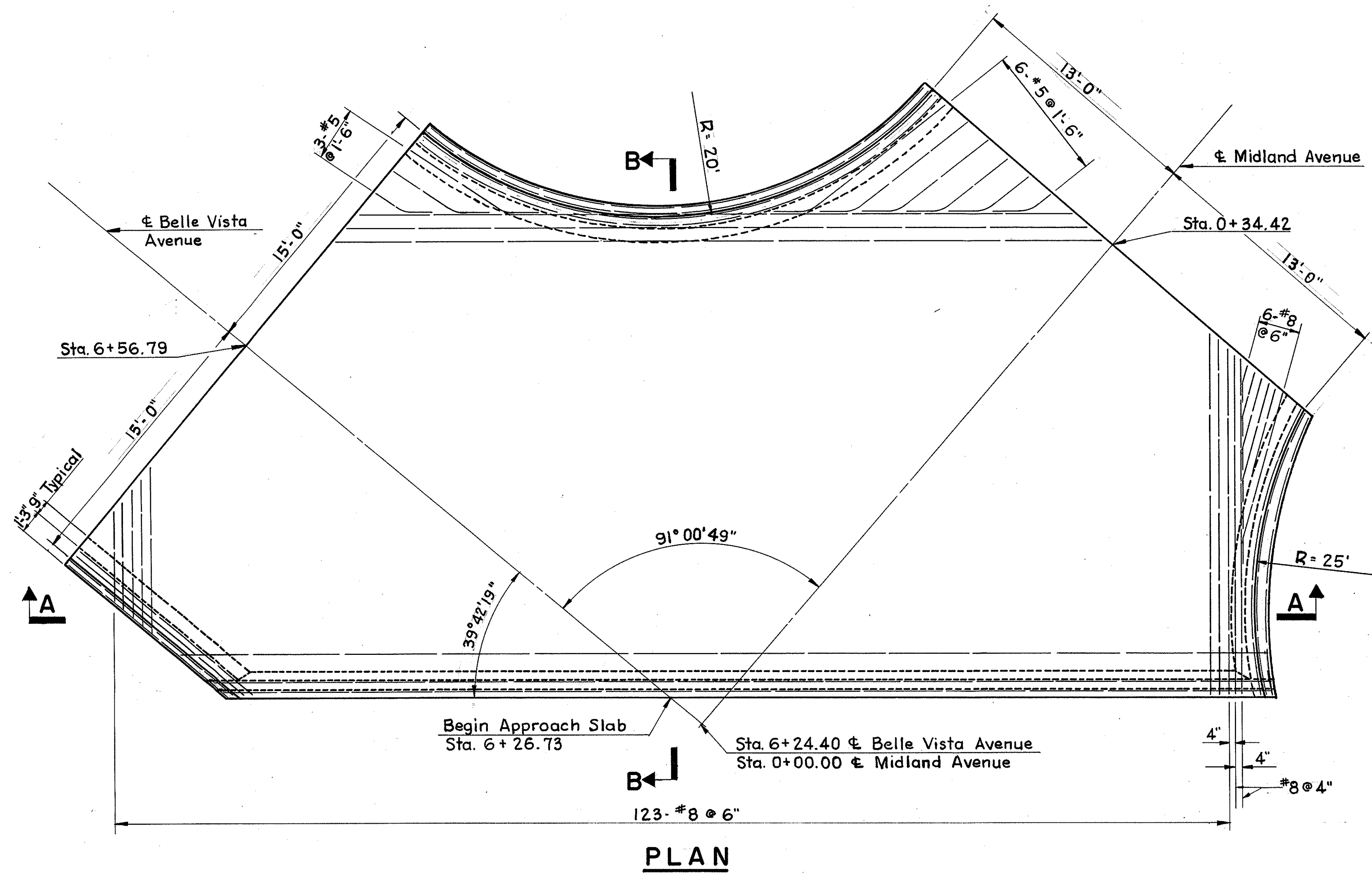
REINFORCING BAR SCHEDULE

BRIDGE NO. MAH-18-1715 UNDER BELLE VISTA AVENUE

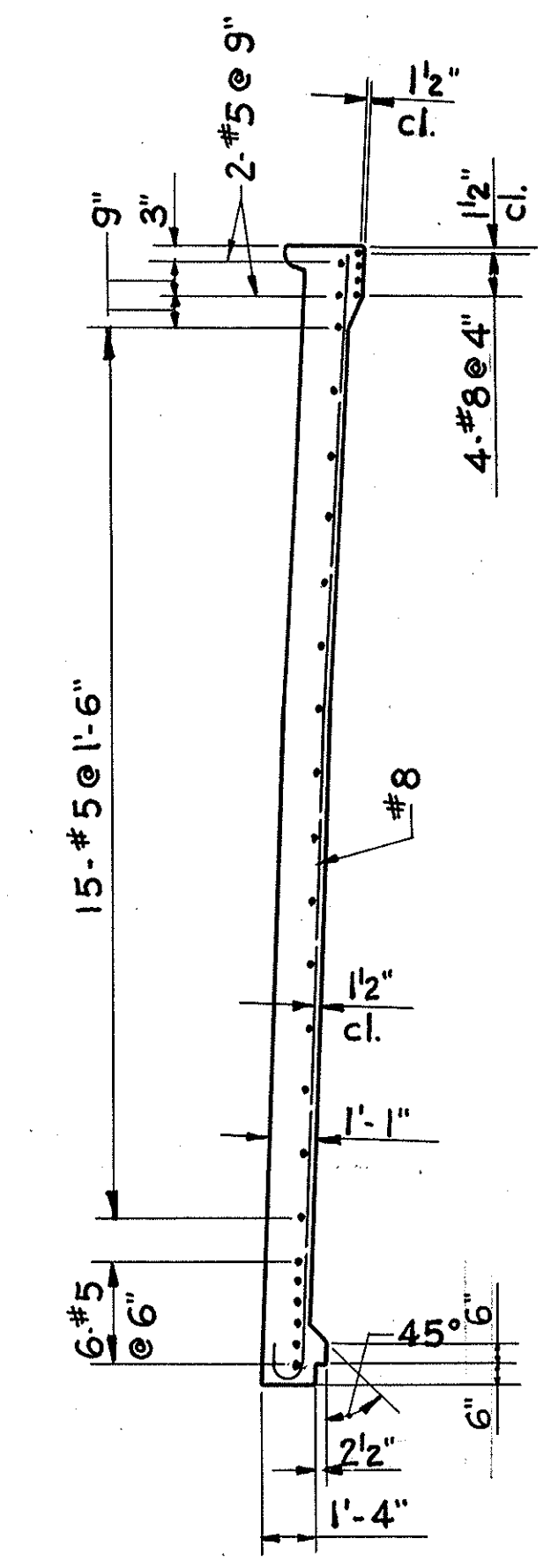
MAHONING COUNTY STA. 606 + 54.21

Table with columns: Designed, Drawn, Traced, Checked, Reviewed, Date, Revised. Values include R.W.K., F.K., and 10-14-63.

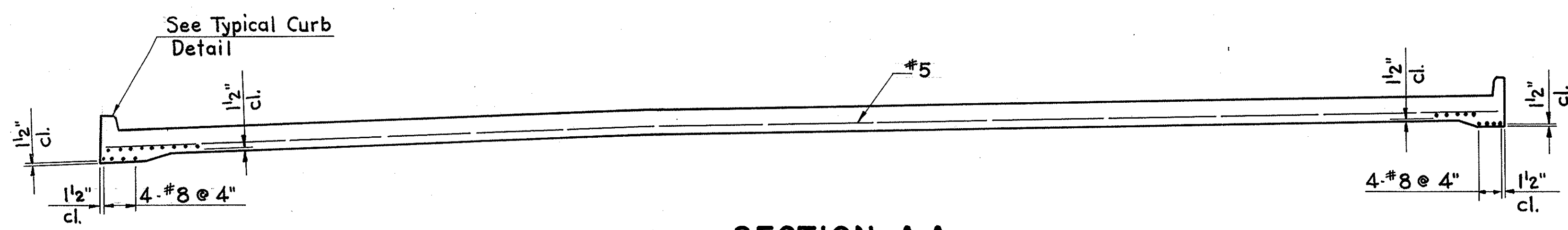
MAHONING COUNTY
MAH-18-15.50



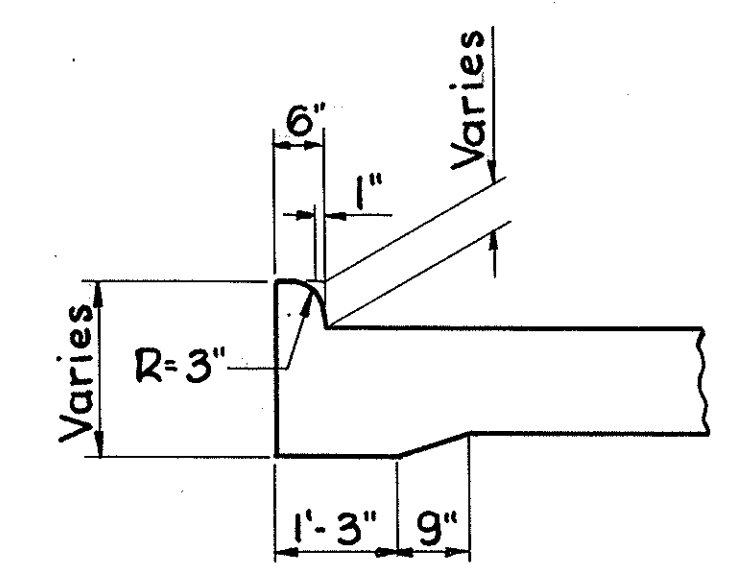
PLAN



SECTION B-B



SECTION A-A



TYPICAL CURB DETAIL

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
APPROACH SLAB					
BRIDGE NO. MAH-18-1715 UNDER BELLE VISTA AVENUE					
MAHONING CO.			STA. 606+54.21		
Designed	Drawn	Traced	Checked	Reviewed	Date
EFR	RC.	R.C.	RLS	F.K.	10-14-63

FED. RD. DIVISION	STATE	PROJECT
2	OHIO	

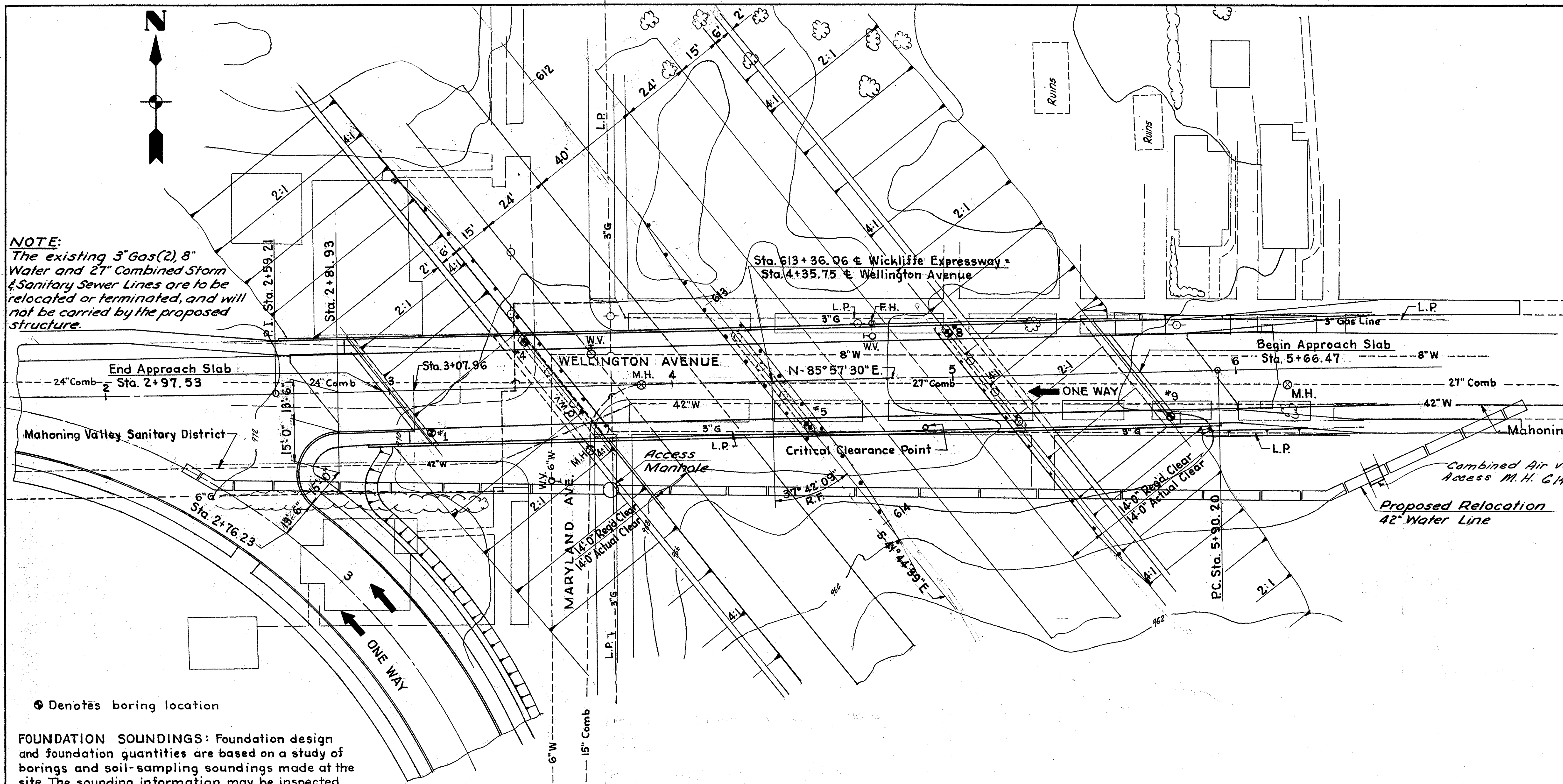
284

**MAH-18-15.50
MAHONING COUNTY**

B.M. 126 - Top stem of hydrant North side of Wellington Avenue 110' E. on Wellington Avenue from Maryland Ave. Elev. 971.75

B.M. 125 - Top stem of hydrant North side of Wellington Avenue 100' W. on Wellington Avenue from Lakeview Ave. Elev. 960.18.

NOTE:
The existing 3" Gas(2), 8" Water and 27" Combined Storm & Sanitary Sewer Lines are to be relocated or terminated, and will not be carried by the proposed structure.



⊙ Denotes boring location

FOUNDATION SOUNDINGS: Foundation design and foundation quantities are based on a study of borings and soil-sampling soundings made at the site. The sounding information may be inspected in the office of the Bureau of Bridges in Columbus, or in the Division Office, but the State assumes no responsibility for the accuracy thereof.

A.D.T. 10,700 (1975) - 16% "B" & "C"

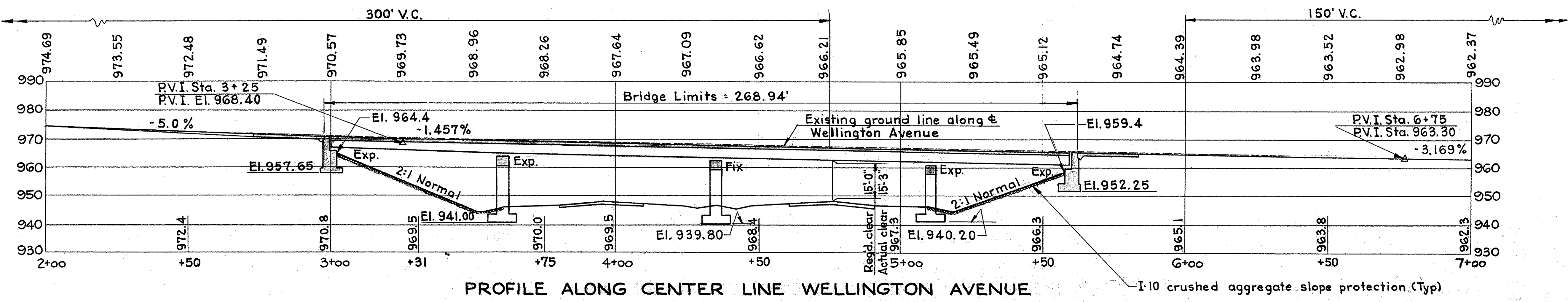
PROPOSED STRUCTURE

TYPE: Continuous steel beam with reinforced concrete deck and substructure
SPANS: 60'-2", 75'-2 1/2", 75'-2 1/2", 52'-8"
ROADWAY: 27'-0" 1/2" 5'-2" sidewalks
LOAD FREQUENCY: C.F. 2000 (57)
SKEW: 37° 42' 09" R.F.
WEARING SURFACE: 1" Monolithic Conc.
APPROACH SLAB: AS-1-54 (25' long)
ALIGNMENT: Tangent

MICHAEL BAKER JR., CONSULTING ENGINEERS
ROCHESTER, PENNSYLVANIA

**SITE PLAN
BRIDGE NO. MAH-18-1730
WELLINGTON AVENUE OVER
WICKLIFFE EXPRESSWAY**

MAHONING COUNTY		STA. 613+36.06	
PRESENT TOPOGRAPHY	DESIGNED		
SURVEYED	DRAWN	CHECKED	REVIEWED
Avrilio	MM	EFR	JH
		RLS	K.A.E.
			8/26-63



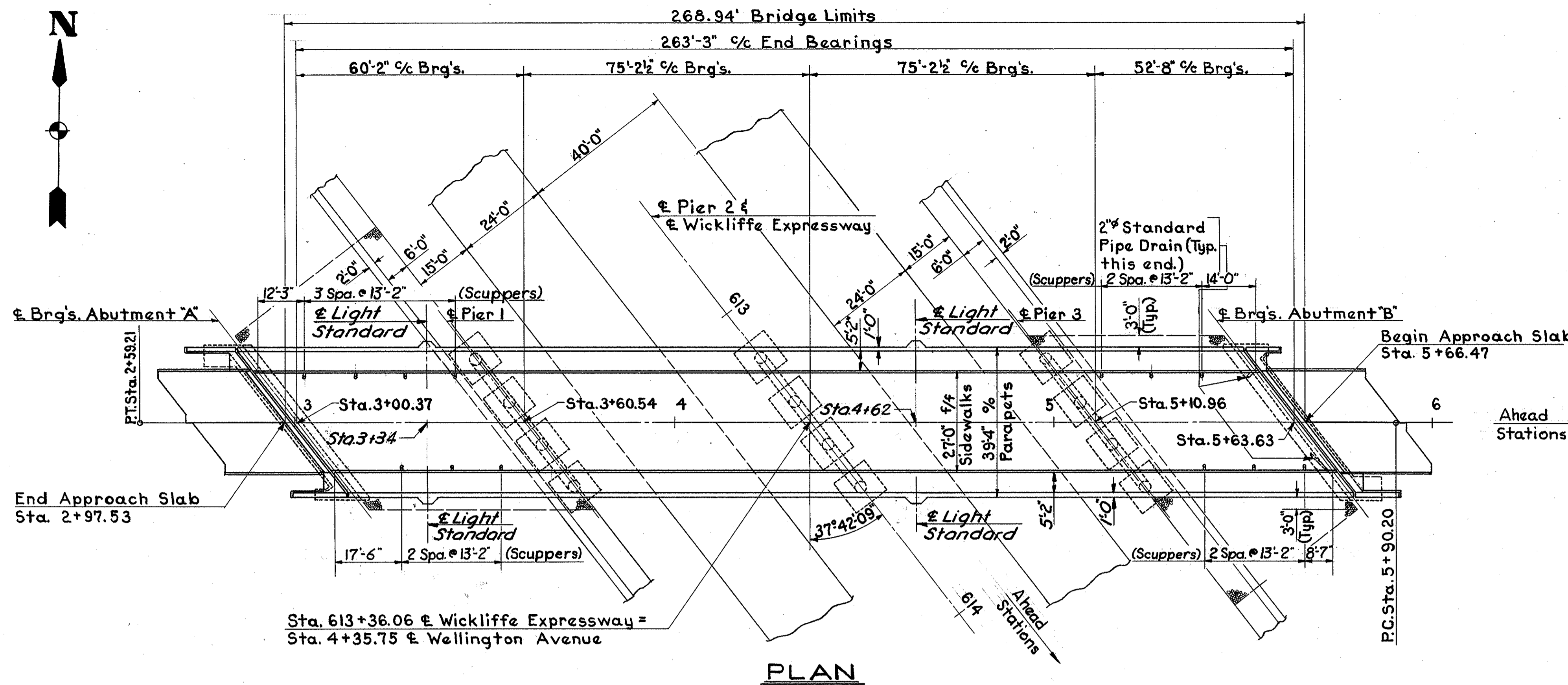
PROFILE ALONG CENTER LINE WELLINGTON AVENUE

1:10 crushed aggregate slope protection (Typ)

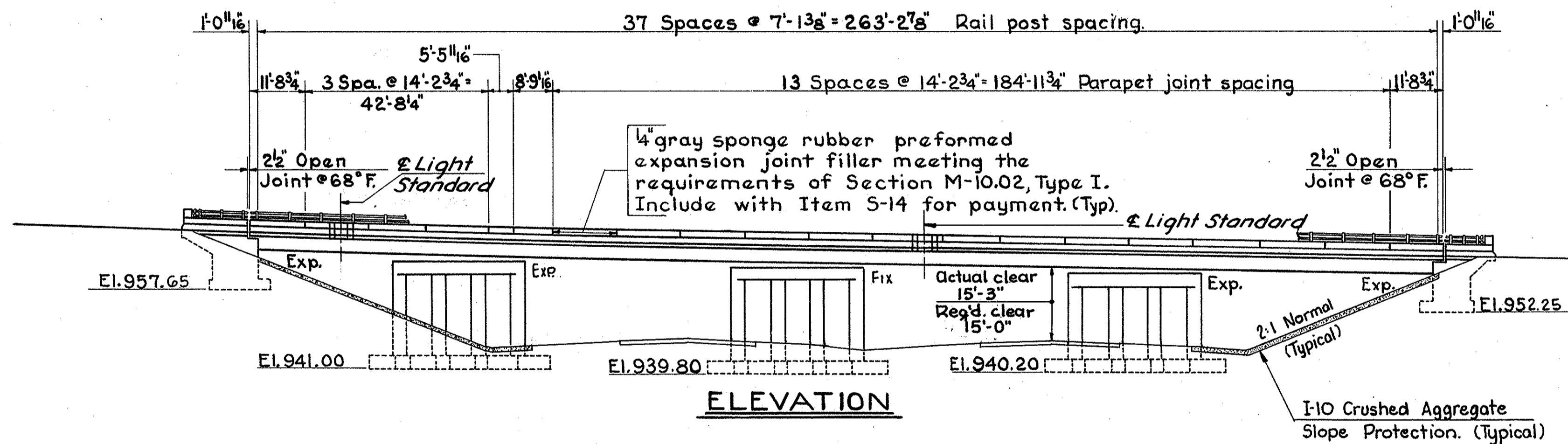
**MAH-18-15.50
MAHONING COUNTY**

GENERAL NOTES

- **DESIGN SPECIFICATIONS:** This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.
- **REFERENCE** shall be made to Standard Drawings CSB-2-56, Sheets 2 and 3 of 6, revised 2-2-59; AR-1-57 revised 4-2-62; FSB-1-62 revised 1-15-63; AS-1-54 revised 7-5-62; SD-2-64 dated 11-25-64.
- **FOUNDATION BEARING PRESSURE:** Abutment footings are designed for a maximum bearing pressure of 2.0 tons per square foot, pier footings for 2.9 tons per square foot.
- **WELDING** of structural steel shall be Class 2" except as otherwise shown. Welds shown as field welds may, at the option of the Contractor, be made in the shop.
- **UTILITY LINES:** All expense involved in relocating the affected utility lines shall be borne by the Owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.
- **MACHINE FINISH:** The concrete bridge deck shall be finished by the use of a finishing machine.
- **REFERENCE** shall be made to Supplemental Specification S-307 dated 10-1-64 and S-101 dated 7-12-62.
- **DESIGN LOADING:** CF 2000(57)
- **CONCRETE CLASS "C":** Basic unit stress 1333 psi.
- **CONCRETE CLASS "E":** Basic unit stress 1133 psi.
- **STRUCTURAL STEEL:** ASTM A36 - basic unit stress 20,000 psi. (ASTM A7 and A373 steel not permitted)
- **REINFORCING STEEL:** ASTM A15, A16, A160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 psi. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 psi.



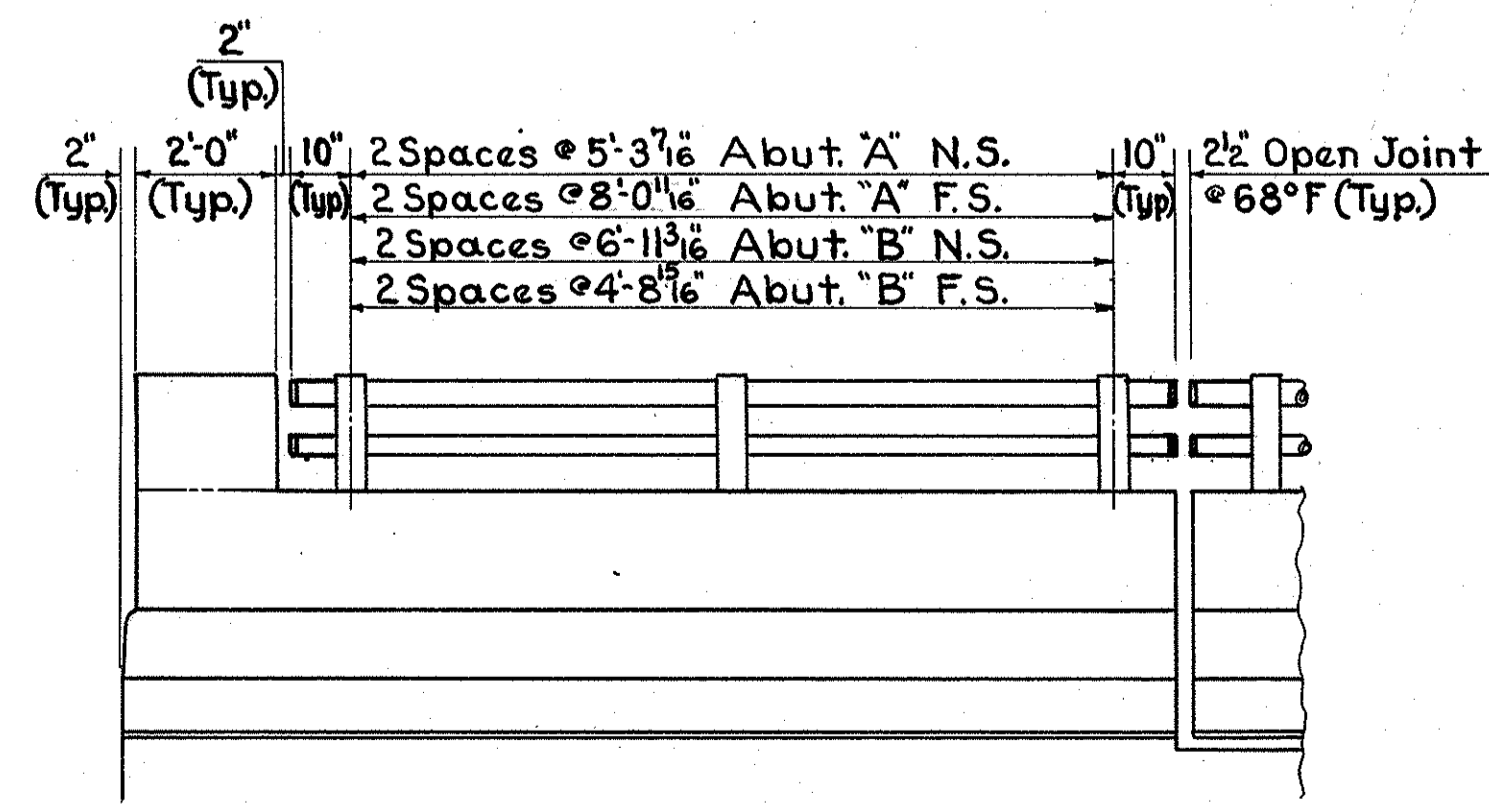
PLAN



ELEVATION

ESTIMATED QUANTITIES							
Item	Total	Unit	Description	Superstr.	Abuts.	Piers	General
E-2	667	cu.yds.	Unclassified excavation		387	280	
S-1	391	cu.yds.	Class "C" concrete, superstructure	391			
S-1	101	cu.yds.	Class "C" concrete, pier caps & columns			101	
S-1	144	cu.yds.	Class "E" concrete, abutments above footings		144		
S-1	171	cu.yds.	Class "E" concrete, footings		64	107	
S-4	150,535	lbs.	Reinforcing steel	100,873	10,926	38,736	
S-7	292,000	lbs.	Structural steel	292,000			
S-8	292,000	lbs.	Field painting of structural steel	292,000			
S-14	596	lin.ft.	Railing Type "C" (aluminum rail and supports, concrete parapet and end posts.)	531	65		
S-29	37	cu.yds.	Porous backfill		37		
S-29	13	each	Scuppers, including supports	13			
I-10	800	sq.yds.	Crushed aggregate slope protection				800
S-101	391	each	Water-reducing, set-retarding admixture	391			
S-25			Electrical lighting system, complete *				

* See General Summary - Lighting Sheet



ABUTMENT RAILING DETAIL

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ROCHESTER, PENNSYLVANIA

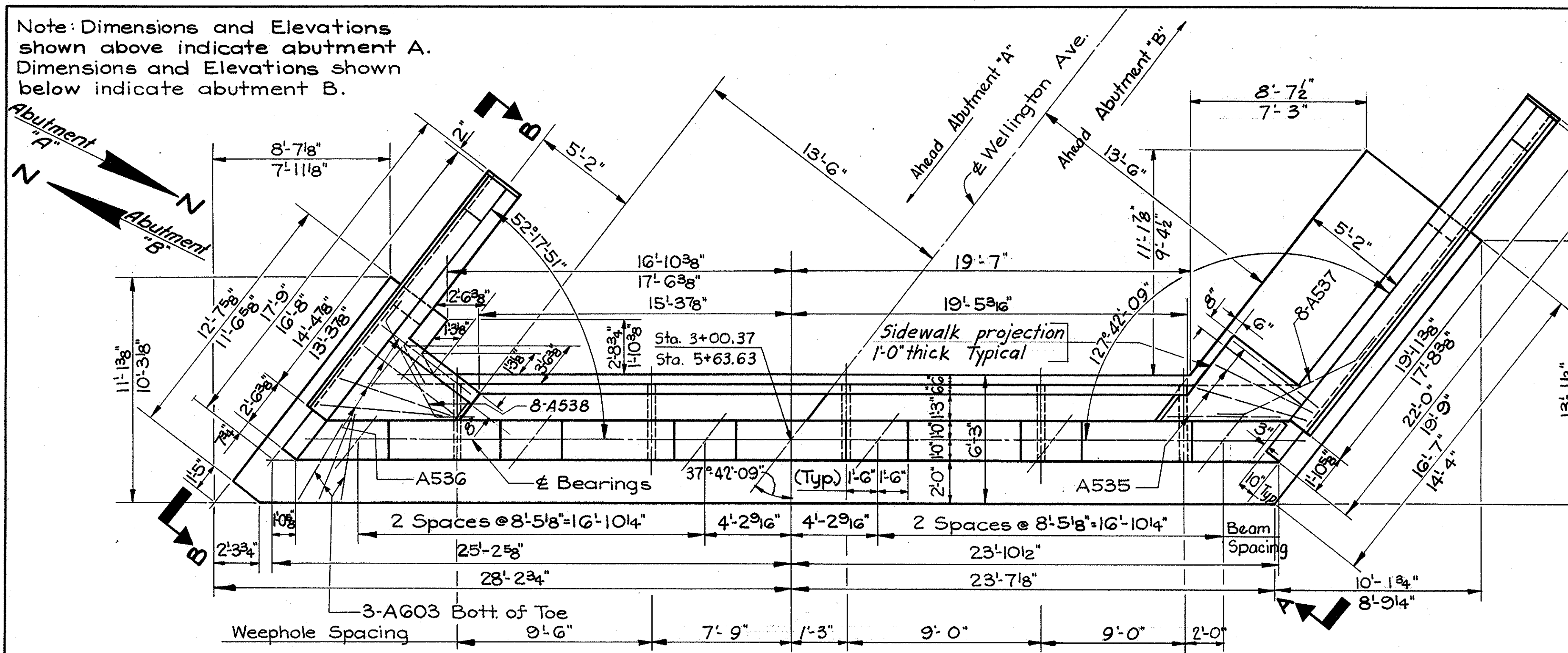
GENERAL PLAN & ELEVATION
BRIDGE NO. MAH-18-1730
WELLINGTON AVENUE OVER
WICKLIFFE EXPRESSWAY

MAHONING COUNTY STA. 613 + 36.06

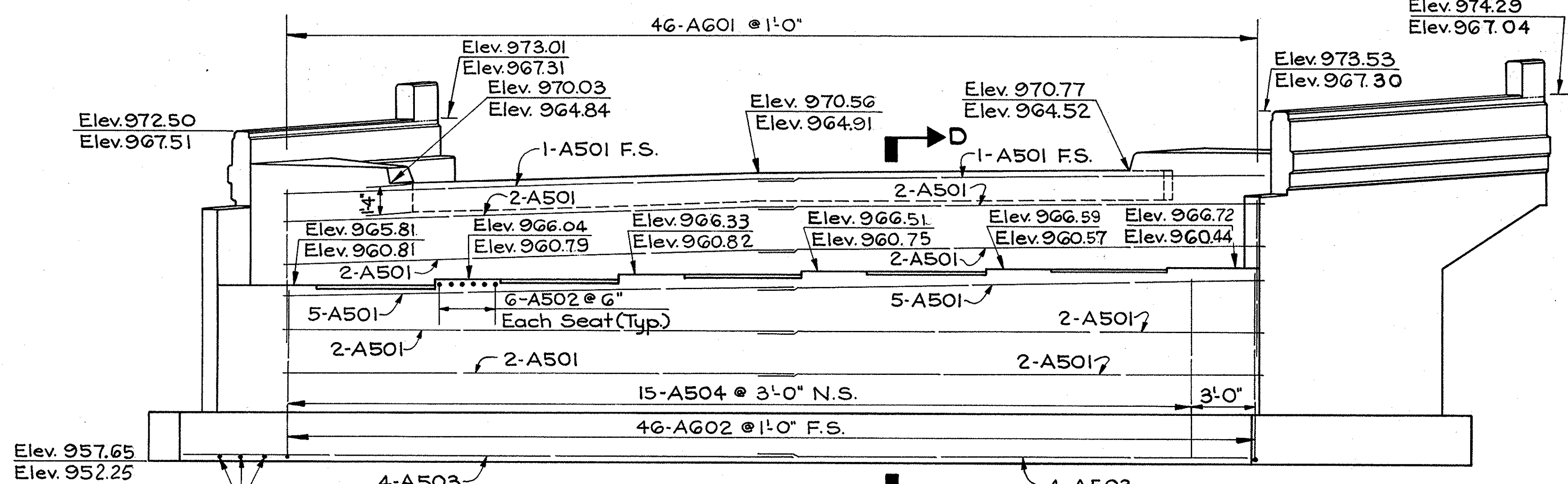
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EPR	EPR	C.B. E.K.	RCS	9/16	8-16-65	

Note: Dimensions and Elevations shown above indicate abutment A. Dimensions and Elevations shown below indicate abutment B.

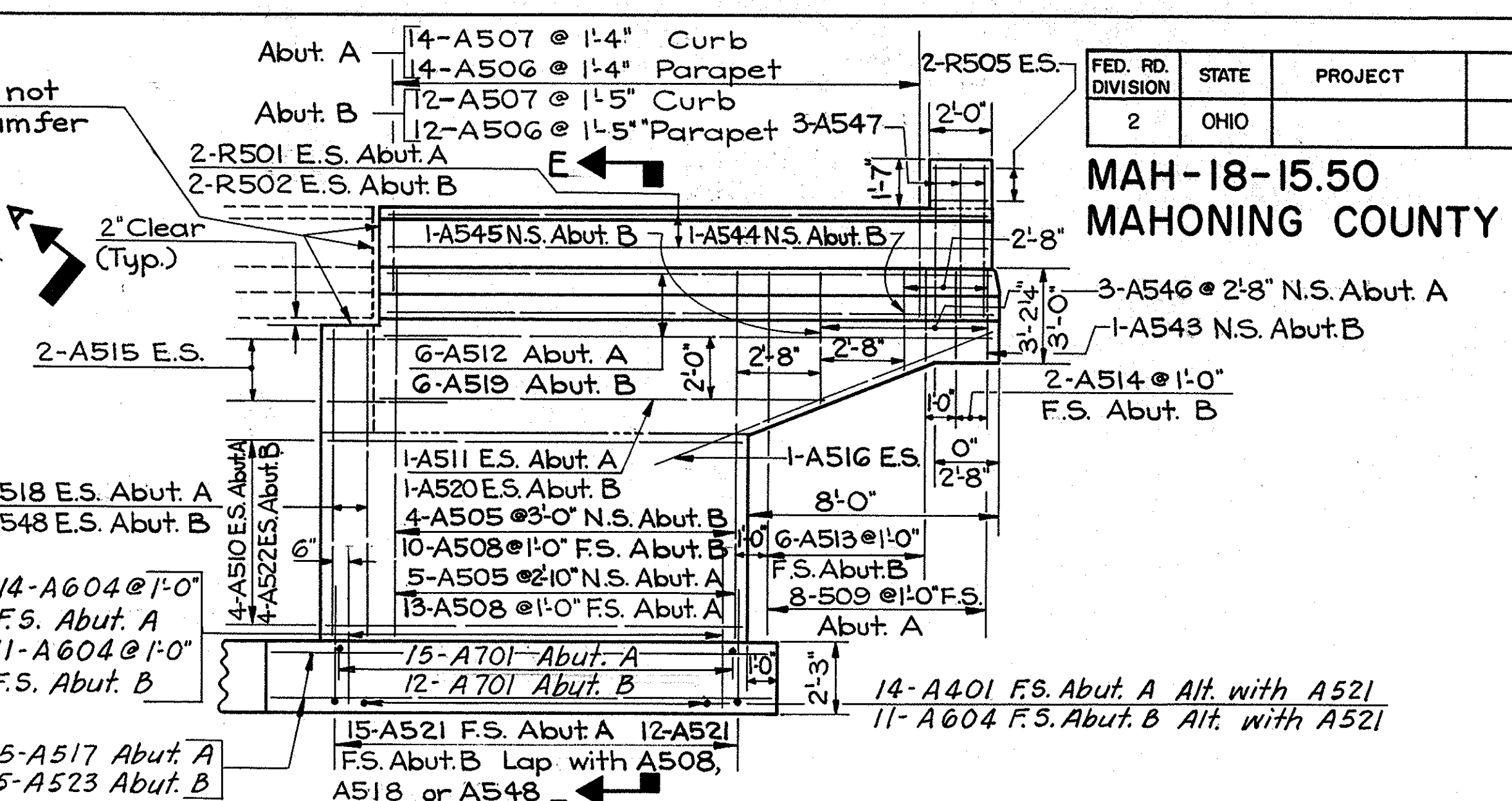
MAH-18-15.50
MAHONING COUNTY



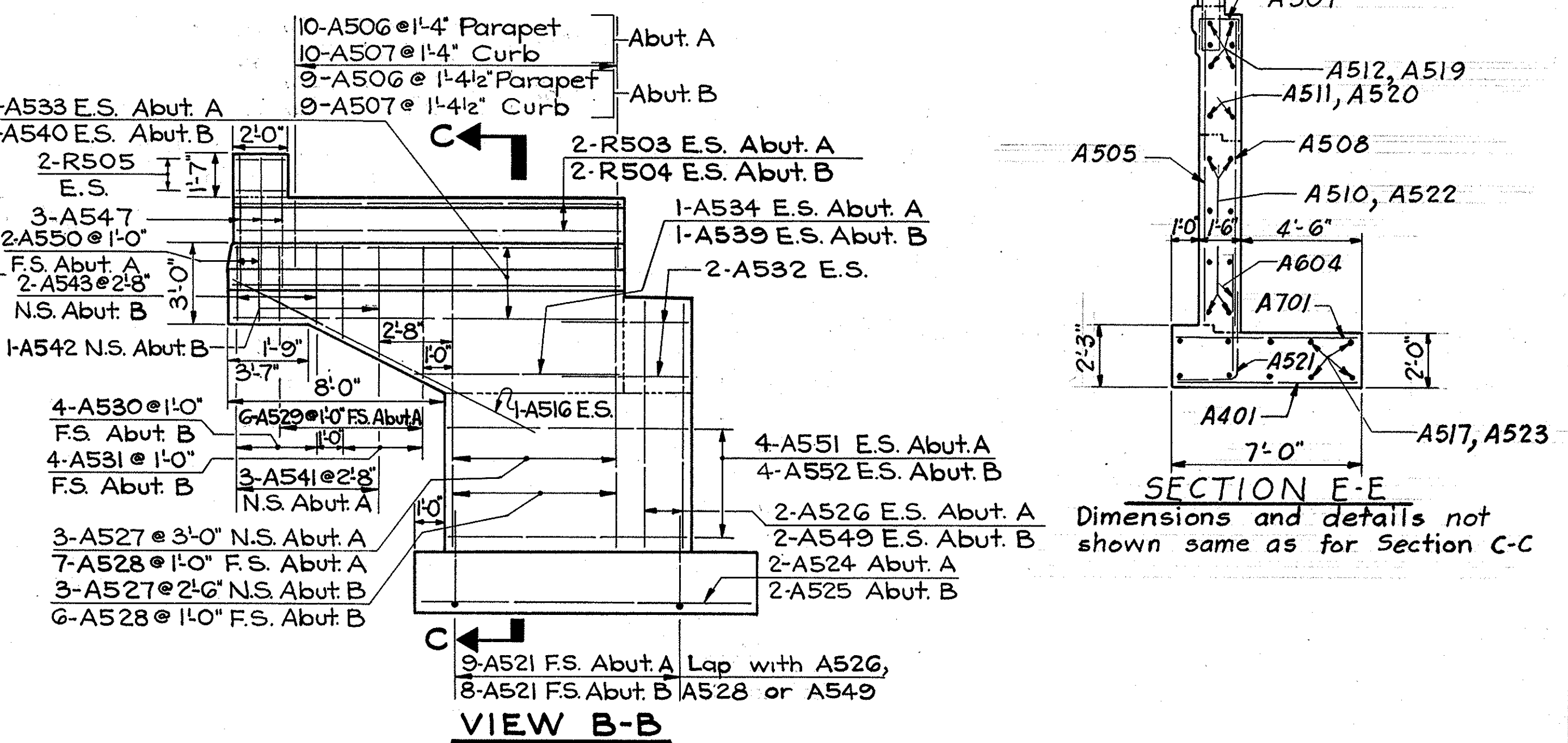
PLAN



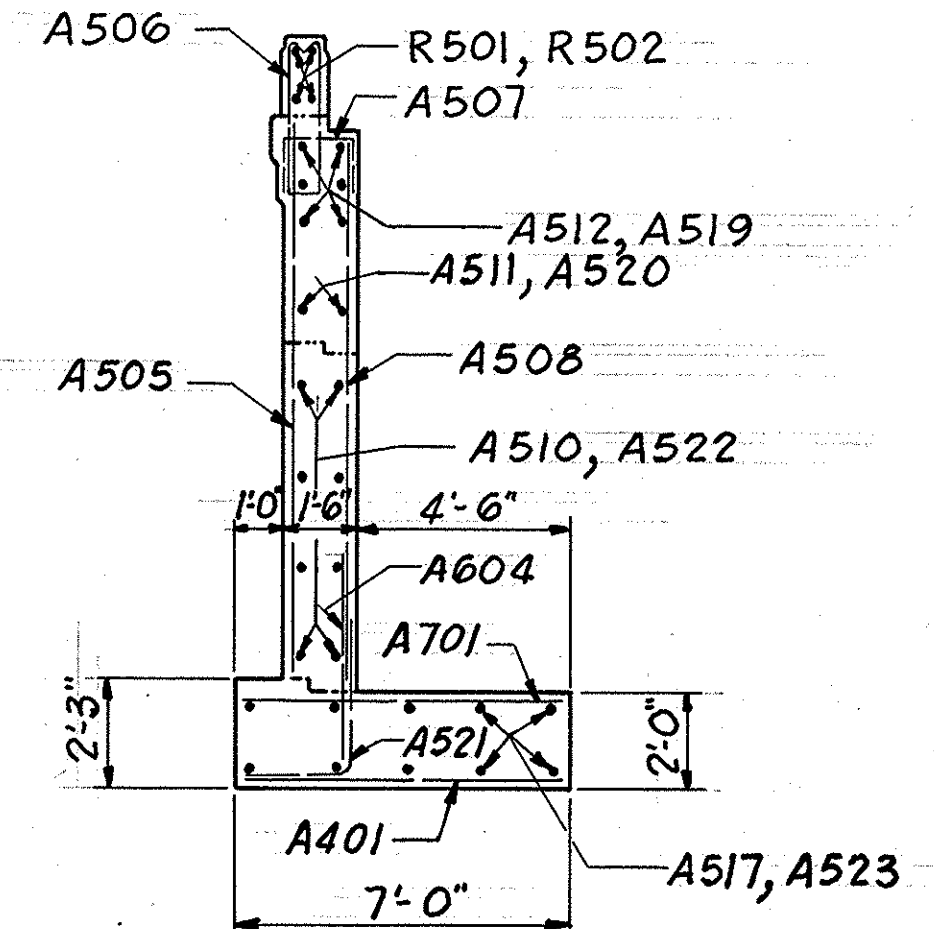
ELEVATION



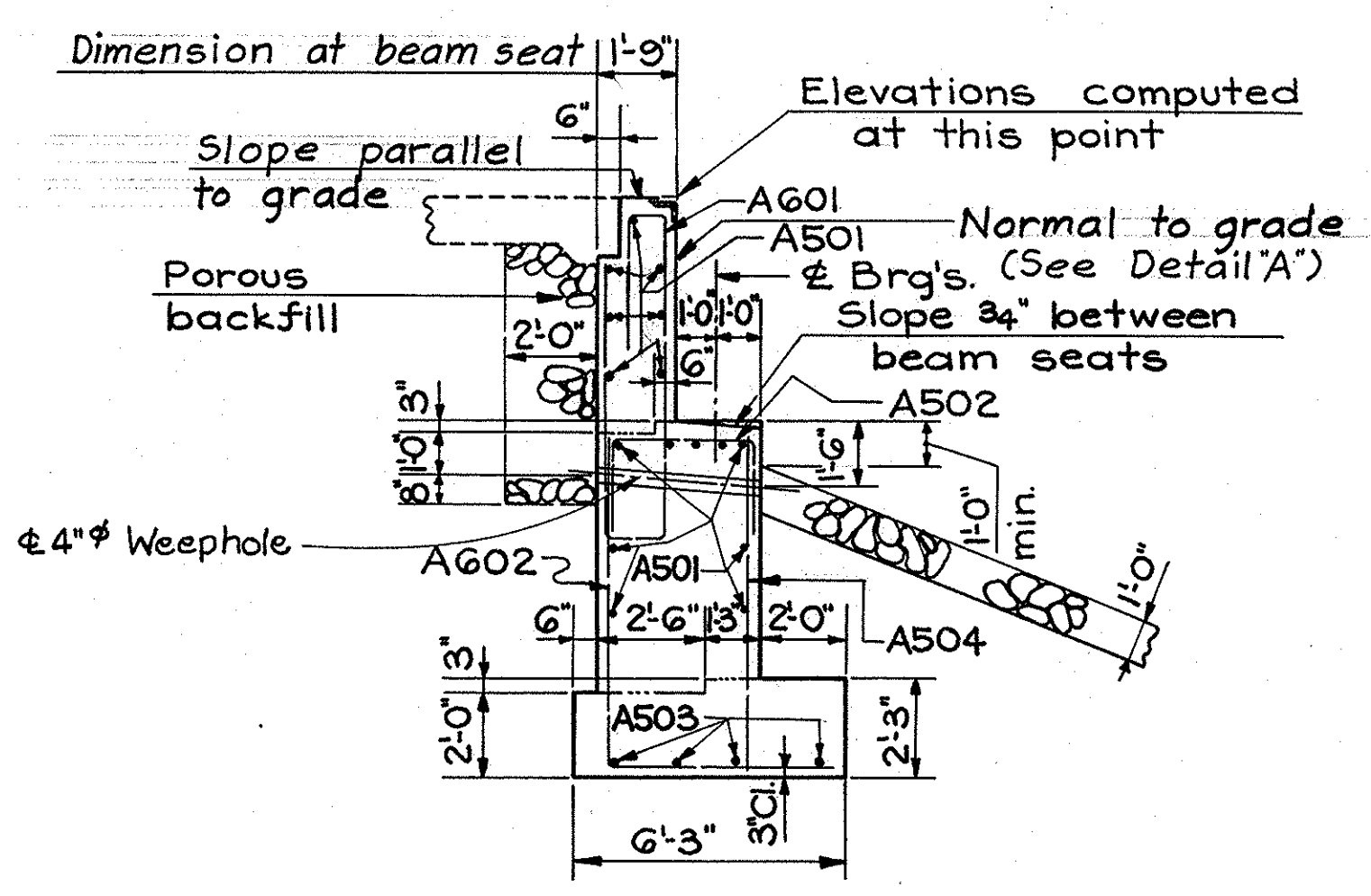
VIEW A-A



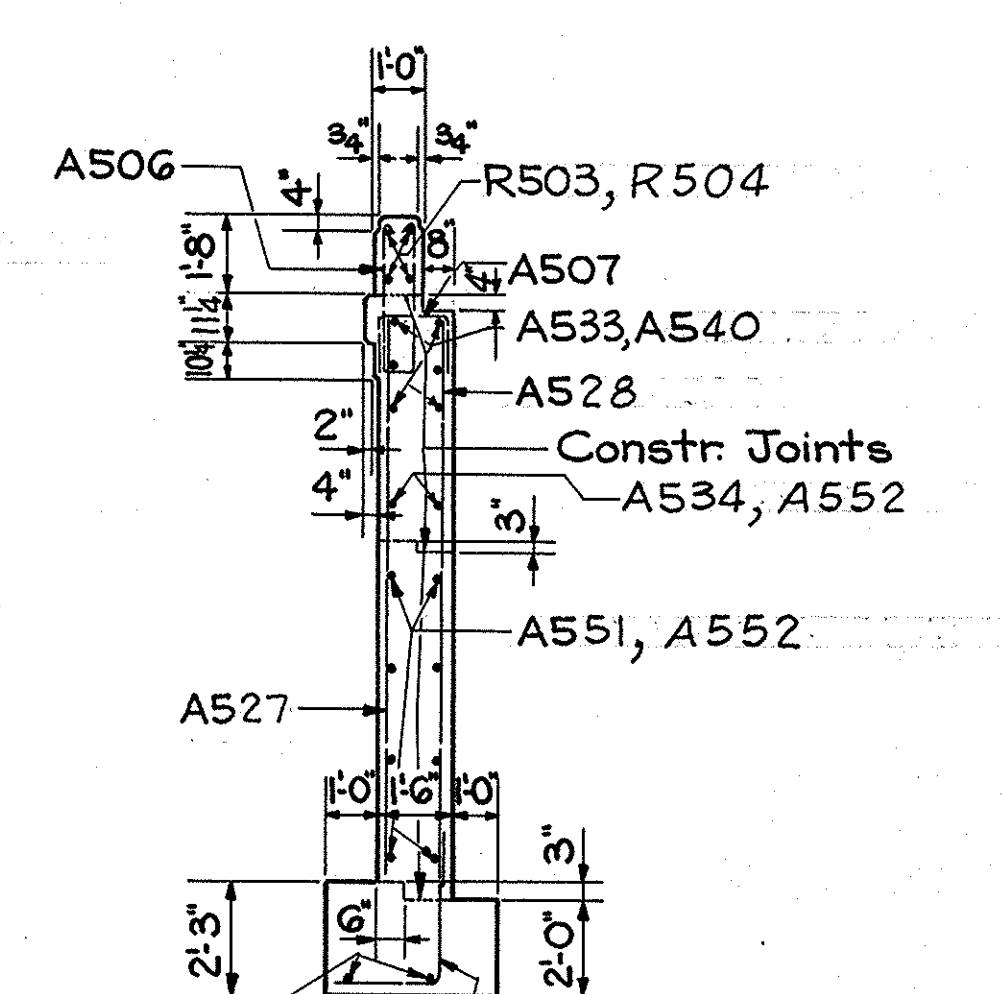
VIEW B-B



SECTION E-E
Dimensions and details not shown same as for Section C-C

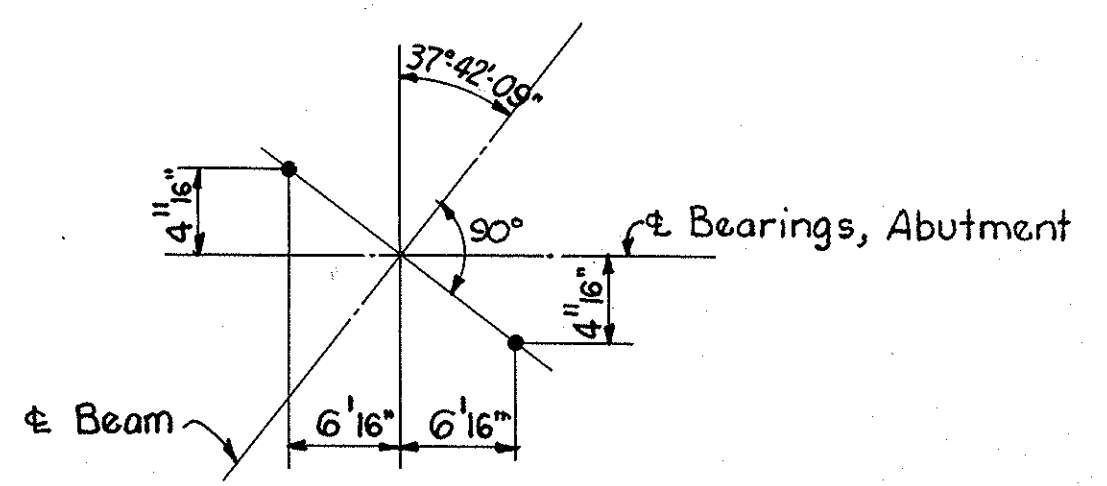


SECTION D-D

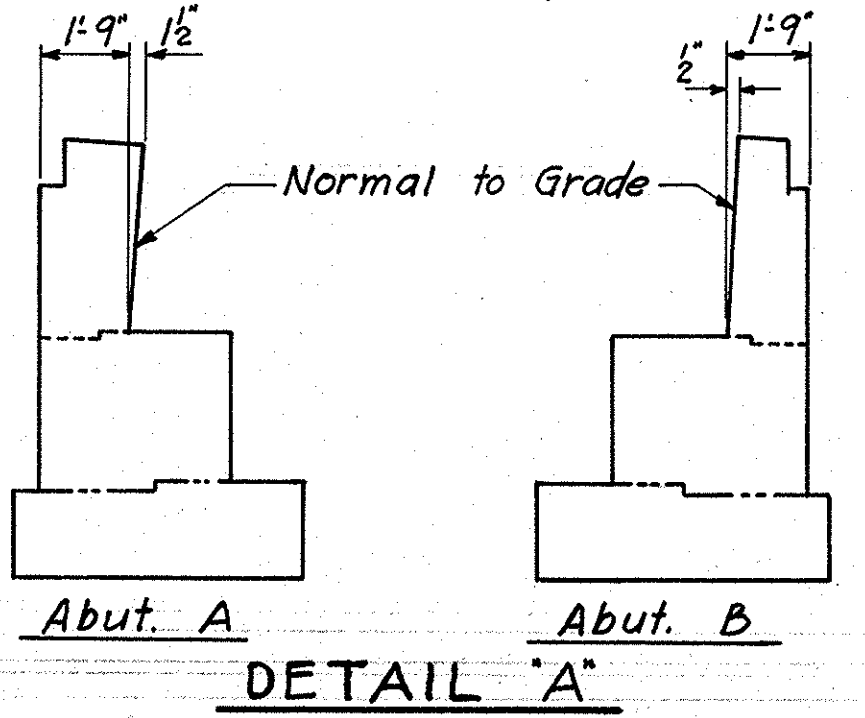


SECTION C-C

LEGEND
N.S. = Near Side
F.S. = Far Side
E.S. = Each Side



ANCHOR ROD LAYOUT
ABUTMENTS "A" & "B"

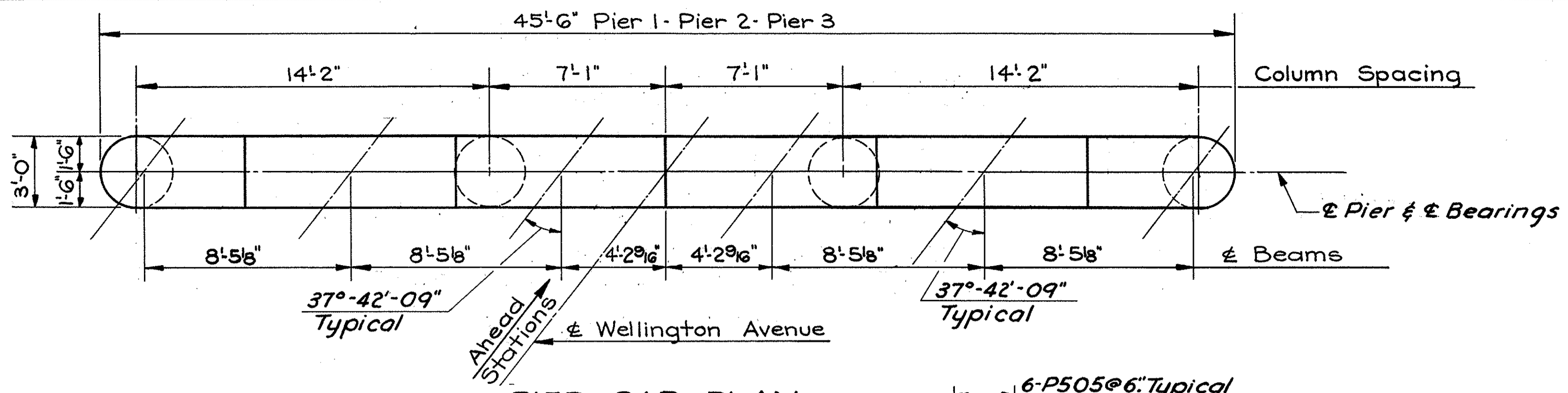


DETAIL 'A'

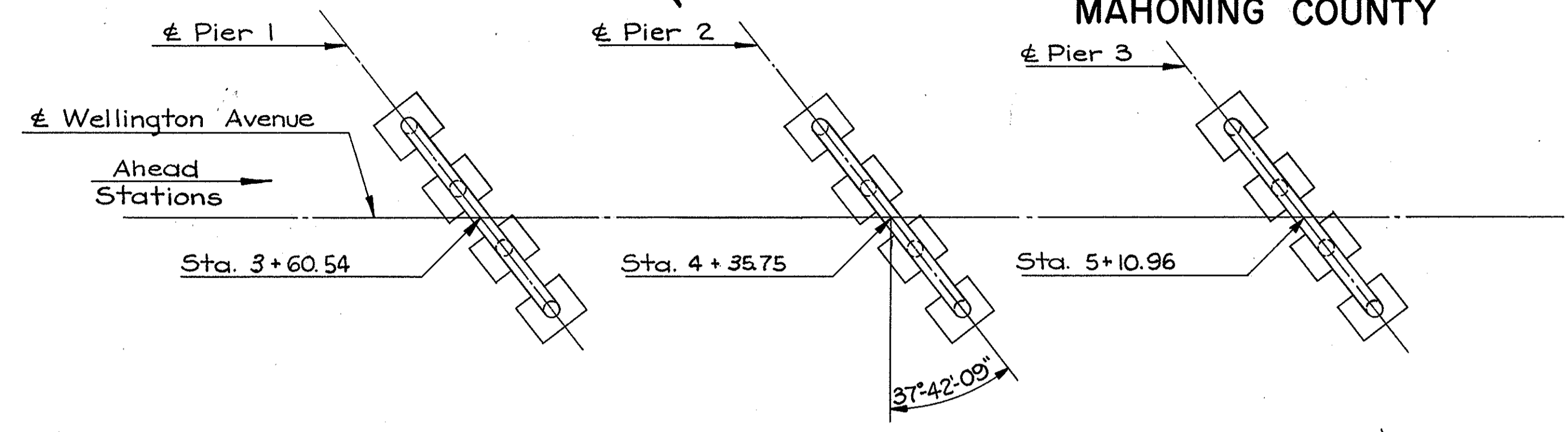
NOTES:
• All abutment concrete shall be Class "E", except parapet concrete which shall be Class "C".
• Concrete and reinforcing steel above parapet construction joint shall be included with Railing for payment.
• Porous backfill, 2 feet thick, shall extend upward to the approach slab and outward to the inside face of wing walls. Excavation therefor, in excess of that required for construction of the abutment, shall be considered as paid for in the bid price per cu yd. paid for porous backfill.

MICHAEL BAKER JR., CONSULTING ENGINEERS ROCHESTER, PENNSYLVANIA					
ABUTMENTS					
BRIDGE NO. MAH-18-1730 WELLINGTON AVENUE OVER WICKLIFFE EXPRESSWAY					
MAHONING COUNTY			STA. 613+36.06		
Designed	Drawn	Traced	Checked	Reviewed	Date
EPR	EPR	J.H.	RLS	FKR	6-16-63

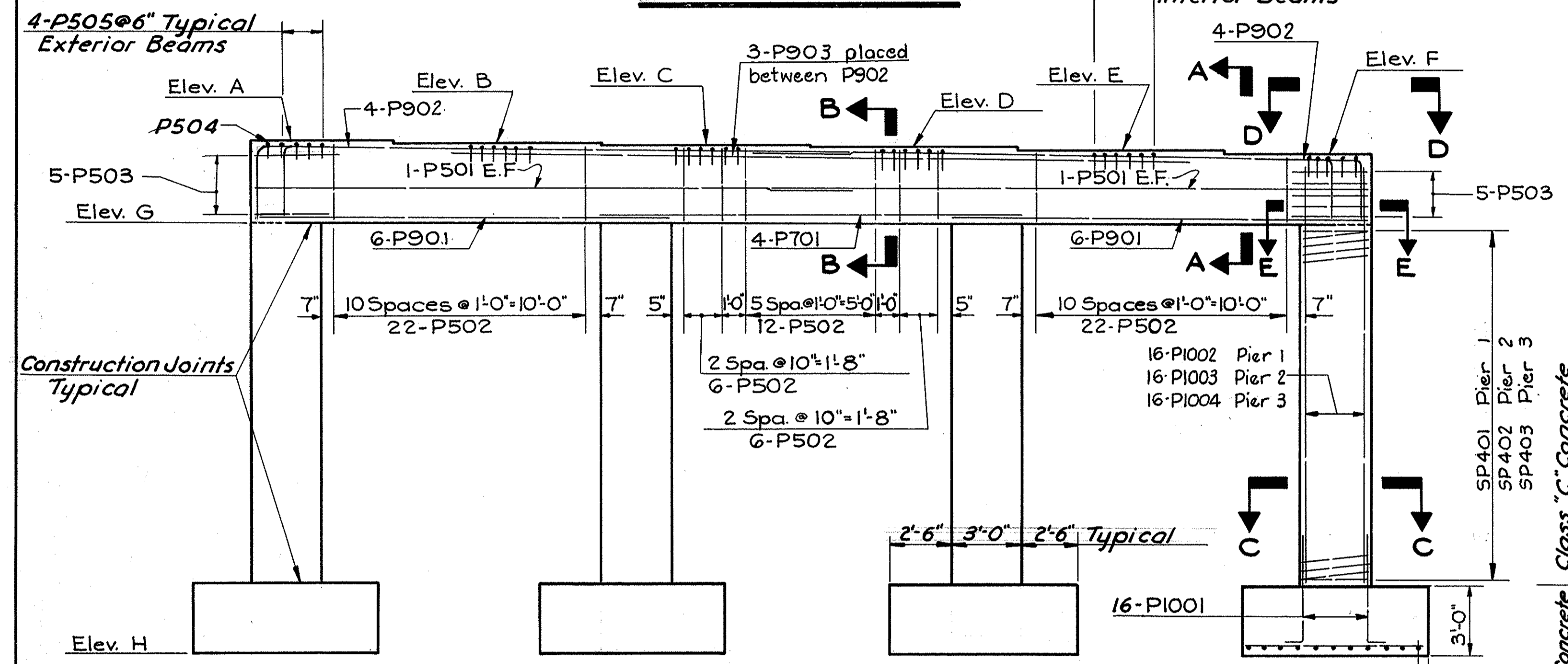
MAH-18-15.50
MAHONING COUNTY



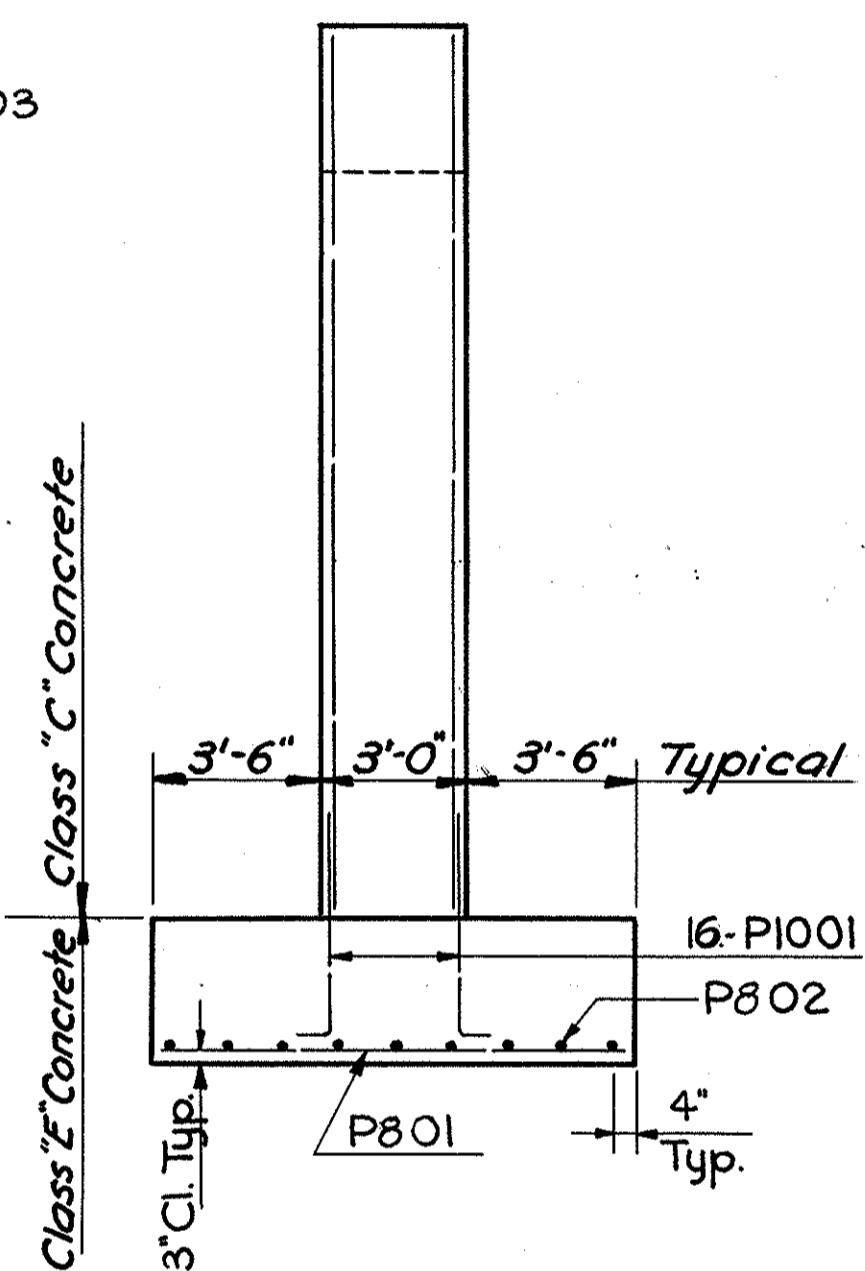
PIER CAP PLAN



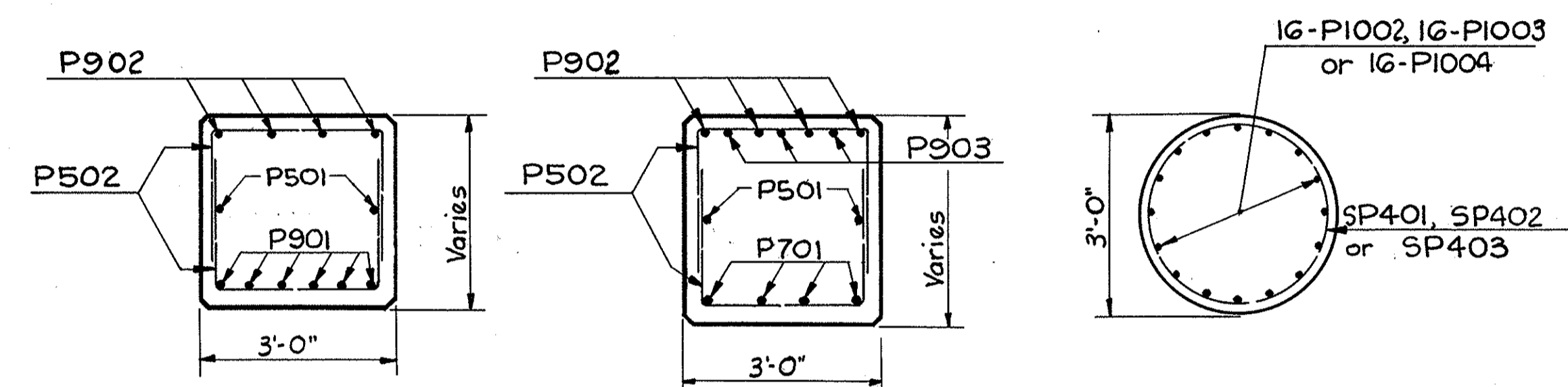
LAYOUT OF PIERS



ELEVATION



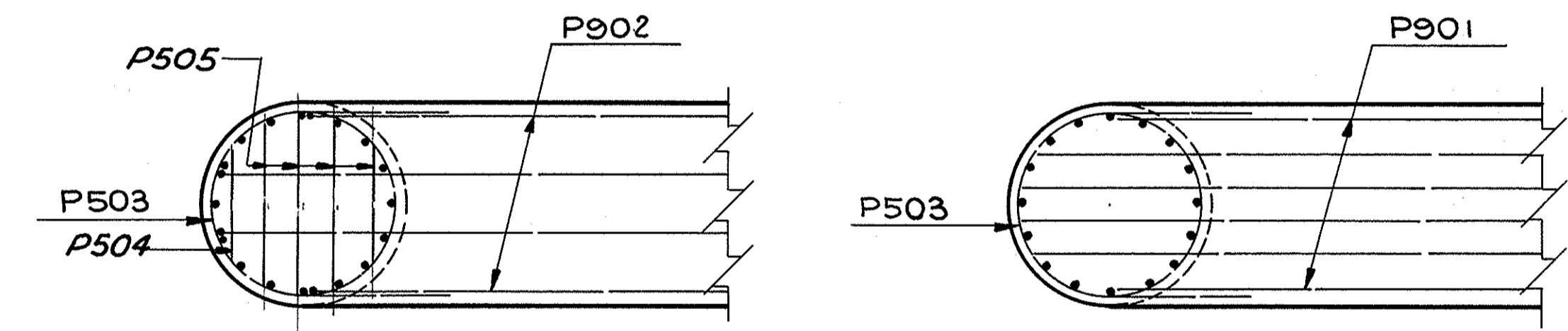
END ELEVATION



SECTION A-A

SECTION B-B

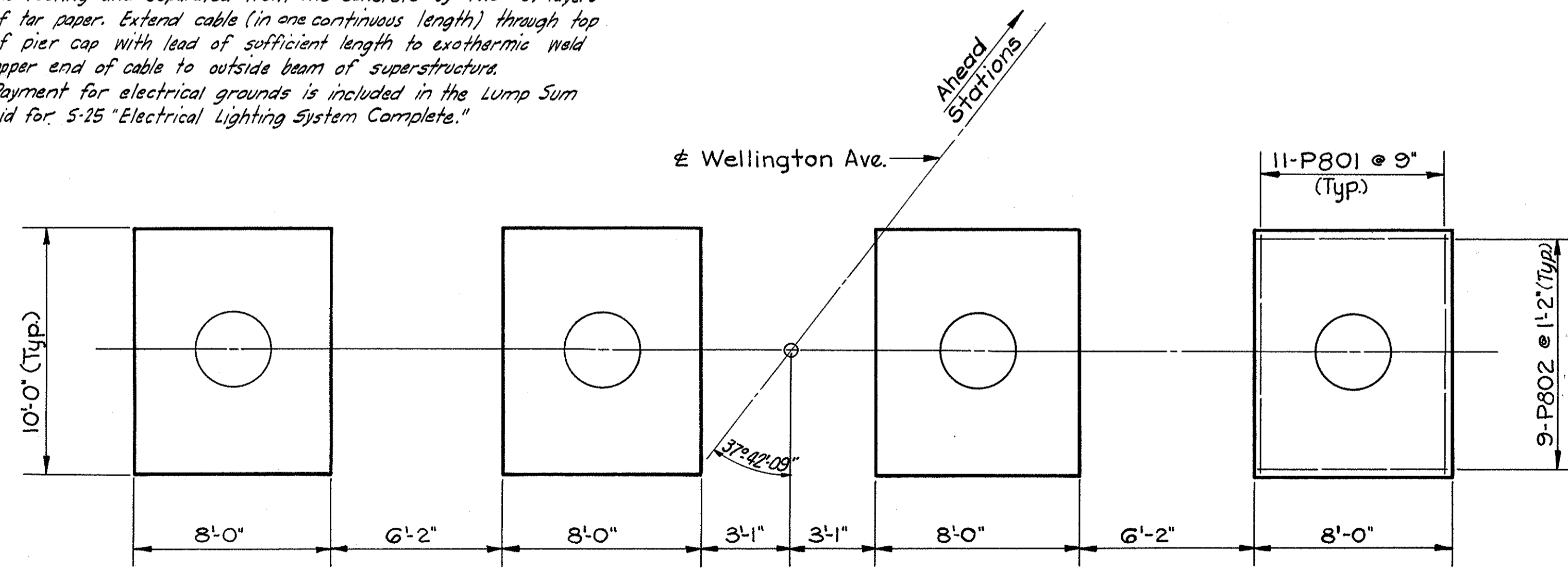
SECTION C-C



VIEW D-D

SECTION E-E

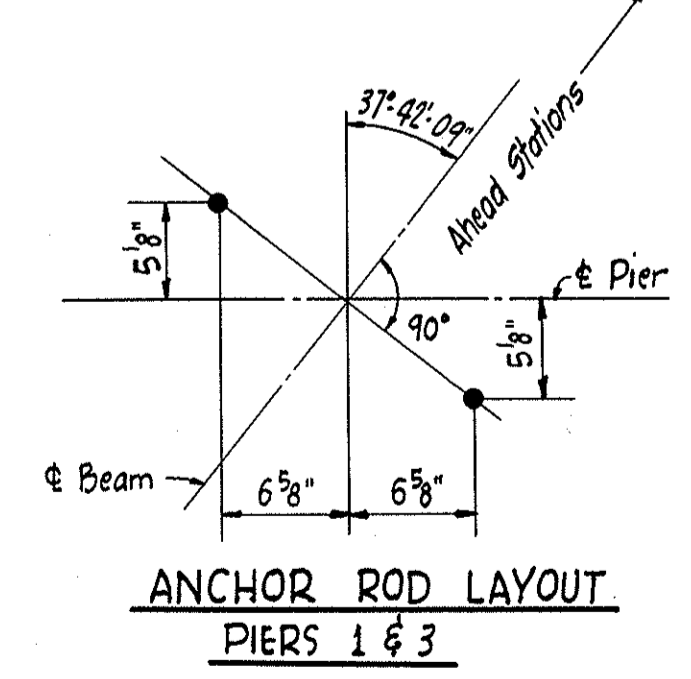
- Notes:
- Superstructure Ground: A No. 10 AWG 7 strand soft annealed bare copper cable shall be encased in the outside column of Pier No. 2. Lower end of cable to extend to the bottom of the pier footing, and an additional 25 ft. length of cable shall be looped under the footing and separated from the concrete by two (2) layers of tar paper. Extend cable (in one continuous length) through top of pier cap with lead of sufficient length to exothermic weld upper end of cable to outside beam of superstructure.
 - Payment for electrical grounds is included in the Lump Sum bid for "5-25 Electrical Lighting System Complete."



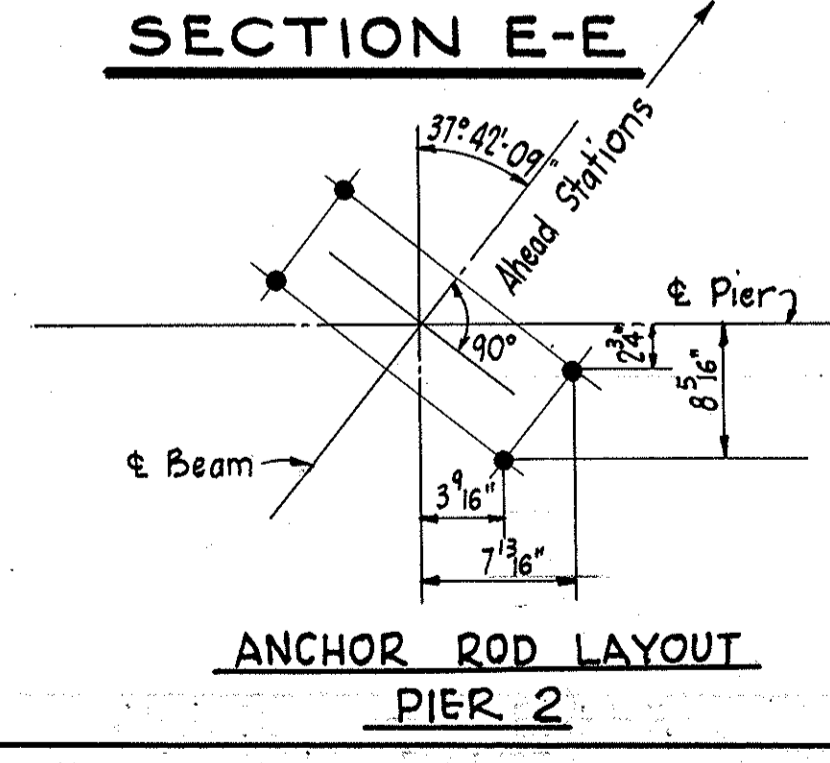
FOOTING PLAN

PIER ELEVATIONS			
Elevations	Pier 1	Pier 2	Pier 3
A	964.68	962.50	961.53
B	964.59	962.45	961.51
C	964.54	962.45	961.54
D	964.40	962.35	961.47
E	964.15	962.15	961.29
F	963.96	962.00	961.16
G	960.96	959.00	958.16
H	941.00	939.80	940.20

NOTE:
Special care shall be taken in placing reinforcing steel in the pier caps so that it will not interfere with the drilling of anchor rod holes.



ANCHOR ROD LAYOUT PIERS 1 & 3



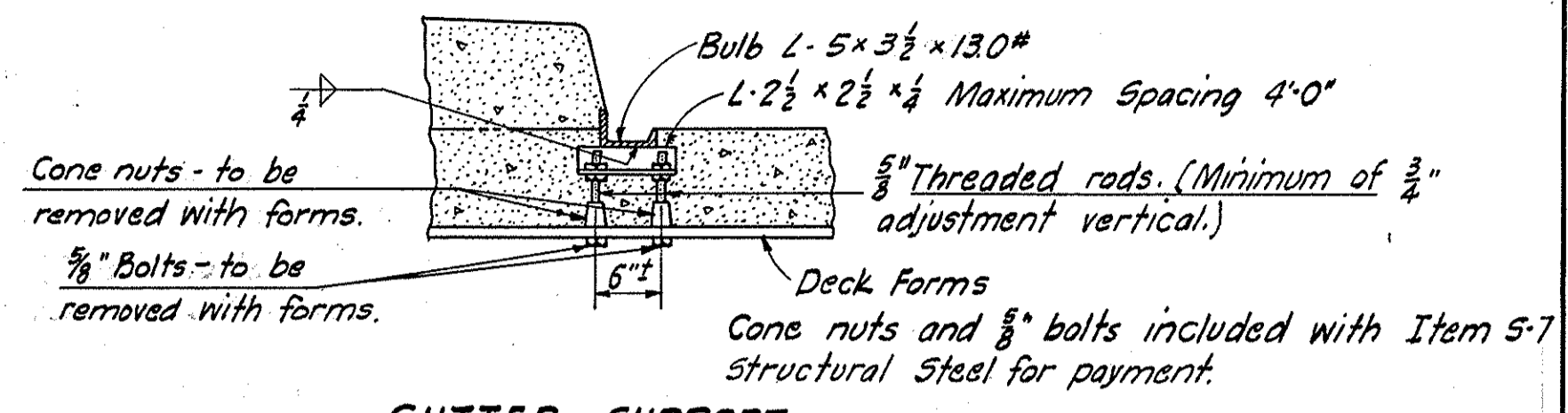
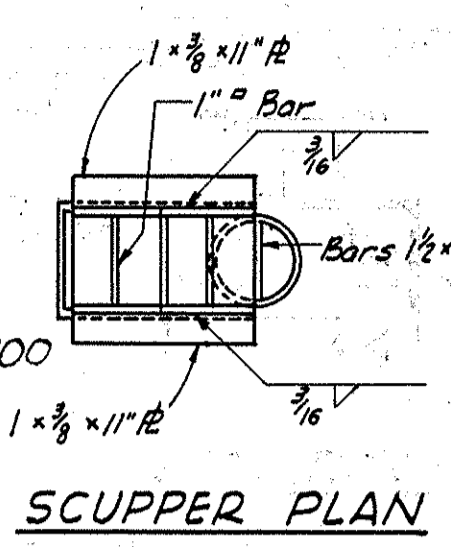
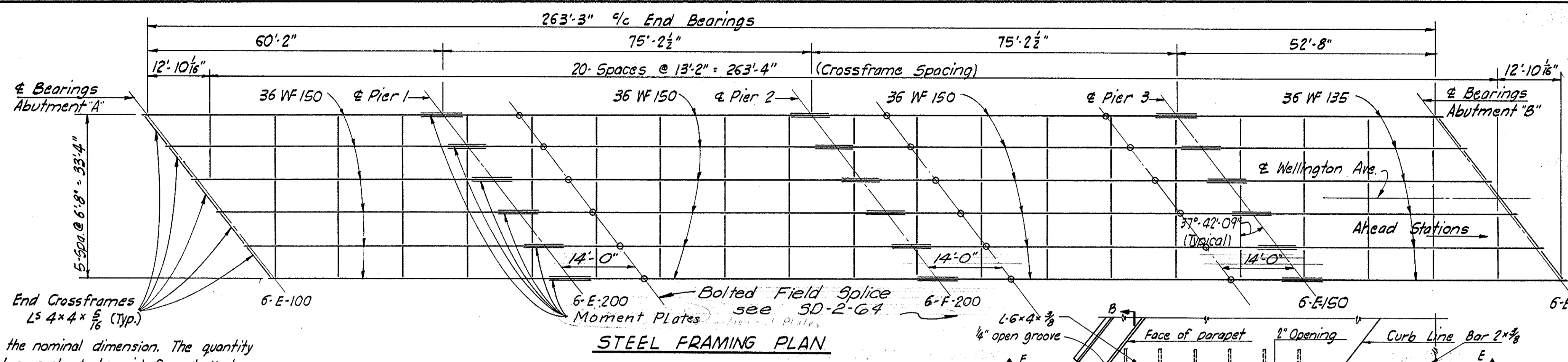
ANCHOR ROD LAYOUT PIER 2

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ROCHESTER, PENNSYLVANIA

PIERS
BRIDGE NO. MAH-18-1730
WELLINGTON AVENUE OVER
WICKLIFFE EXPRESSWAY

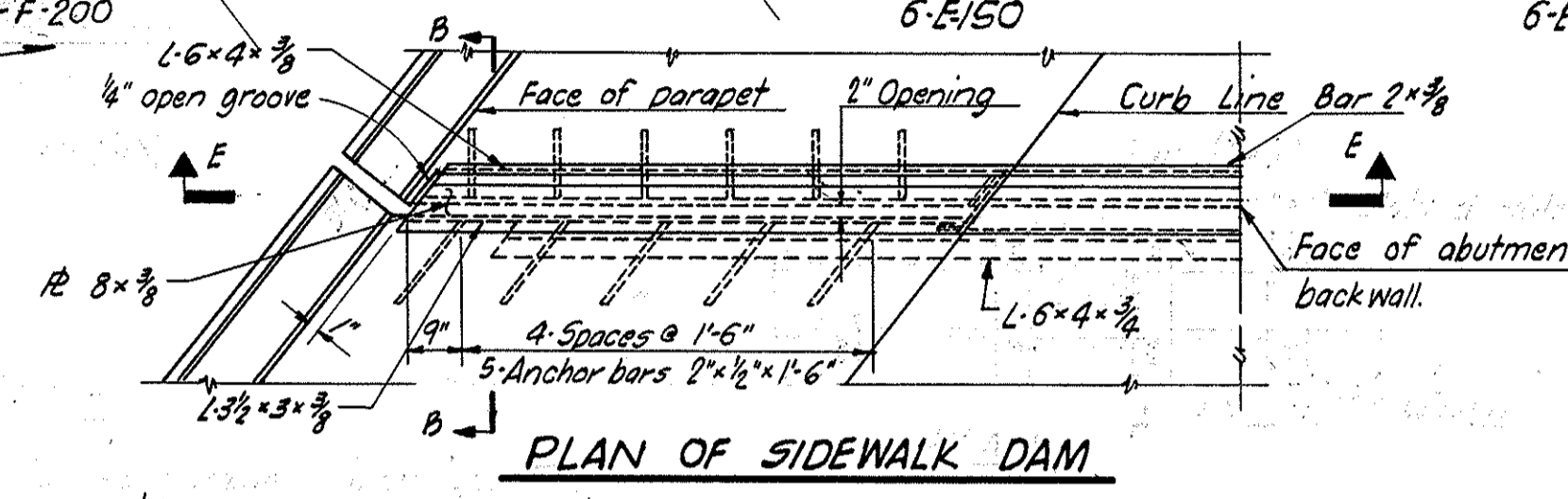
MAHONING COUNTY		STA. 613 + 36.06	
Designed	Drawn	Traced	Checked
Reviewed	Date	Reviewed	Date
DYN	DYN	J.H.	RLS
		8-16-63	

**MAH-18-15.50
MAHONING COUNTY**

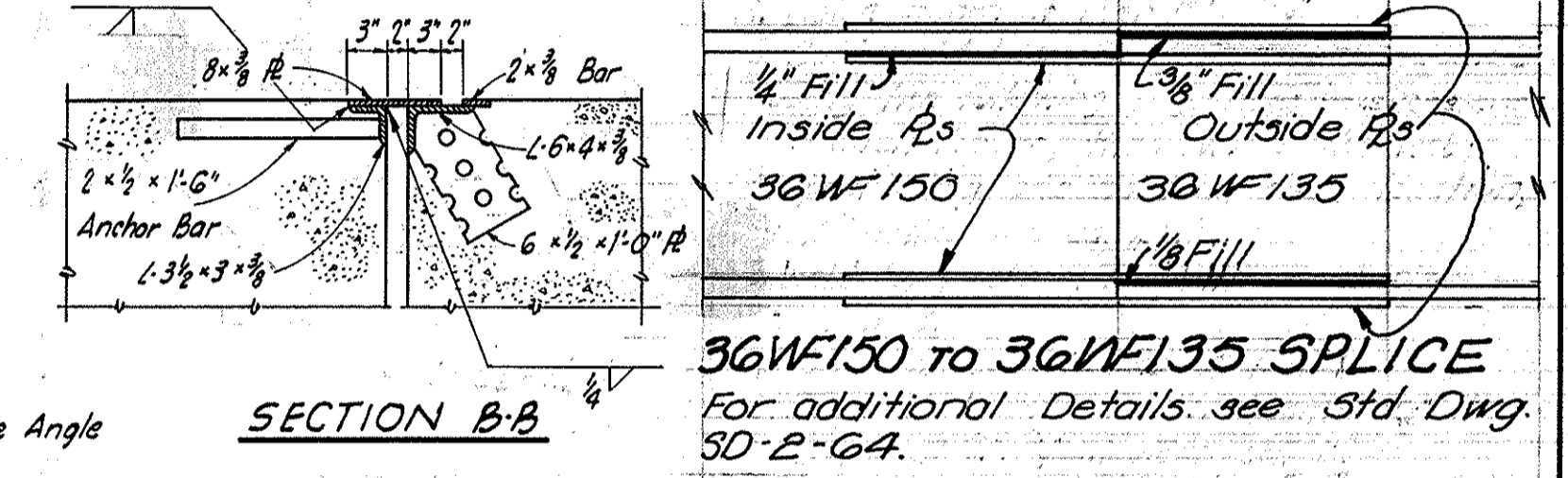
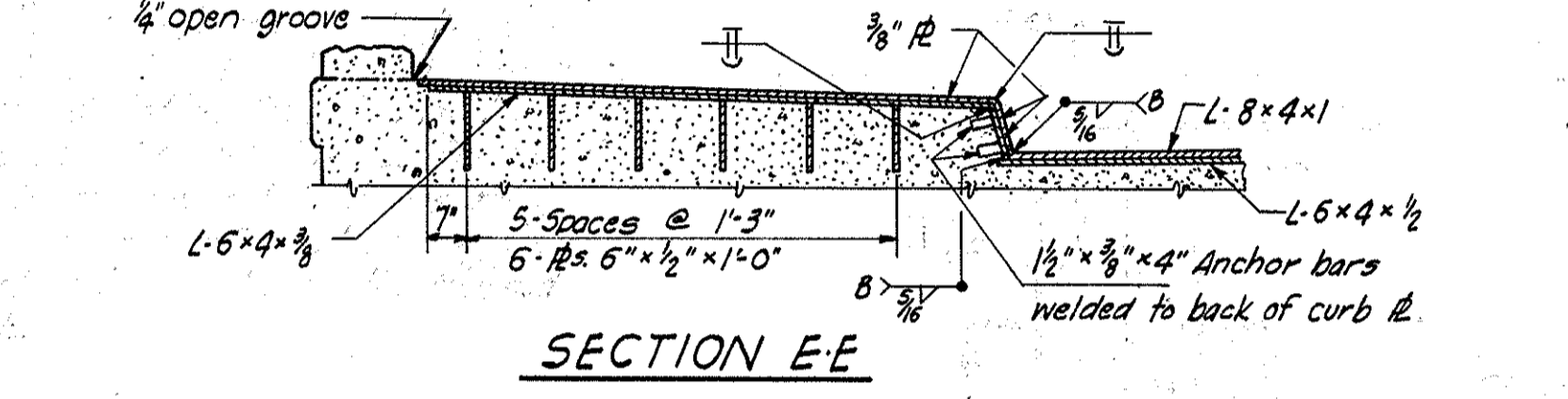
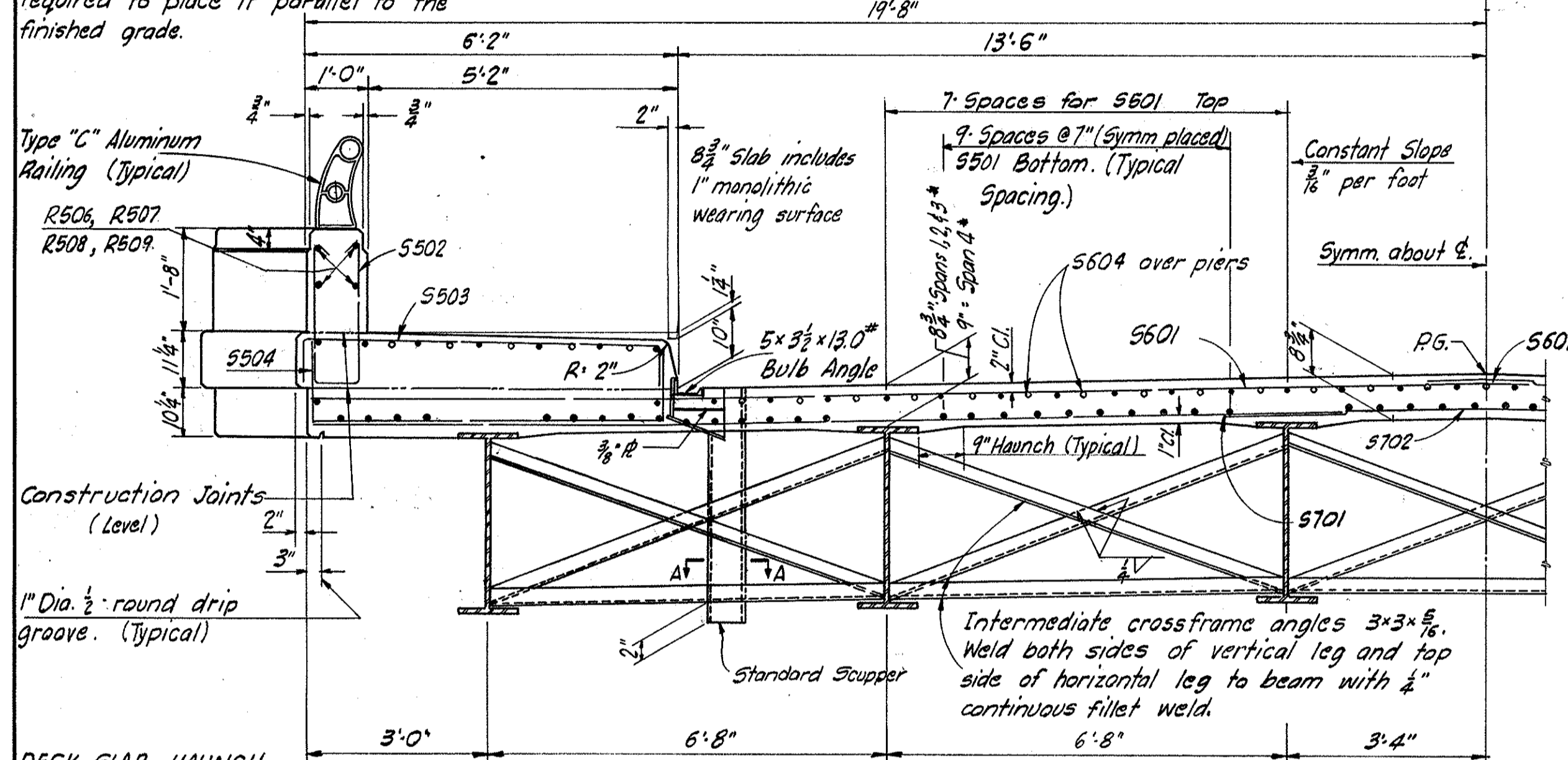


This is the nominal dimension. The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade.

Class "C" Concrete in parapet and reinforcing steel above parapet construction joint included with railing for payment.



Gutters shall be accurately adjusted for alignment and grade, with allowance for dead load deflection before concrete is placed. Gutters shall not be used to support finishing machines.

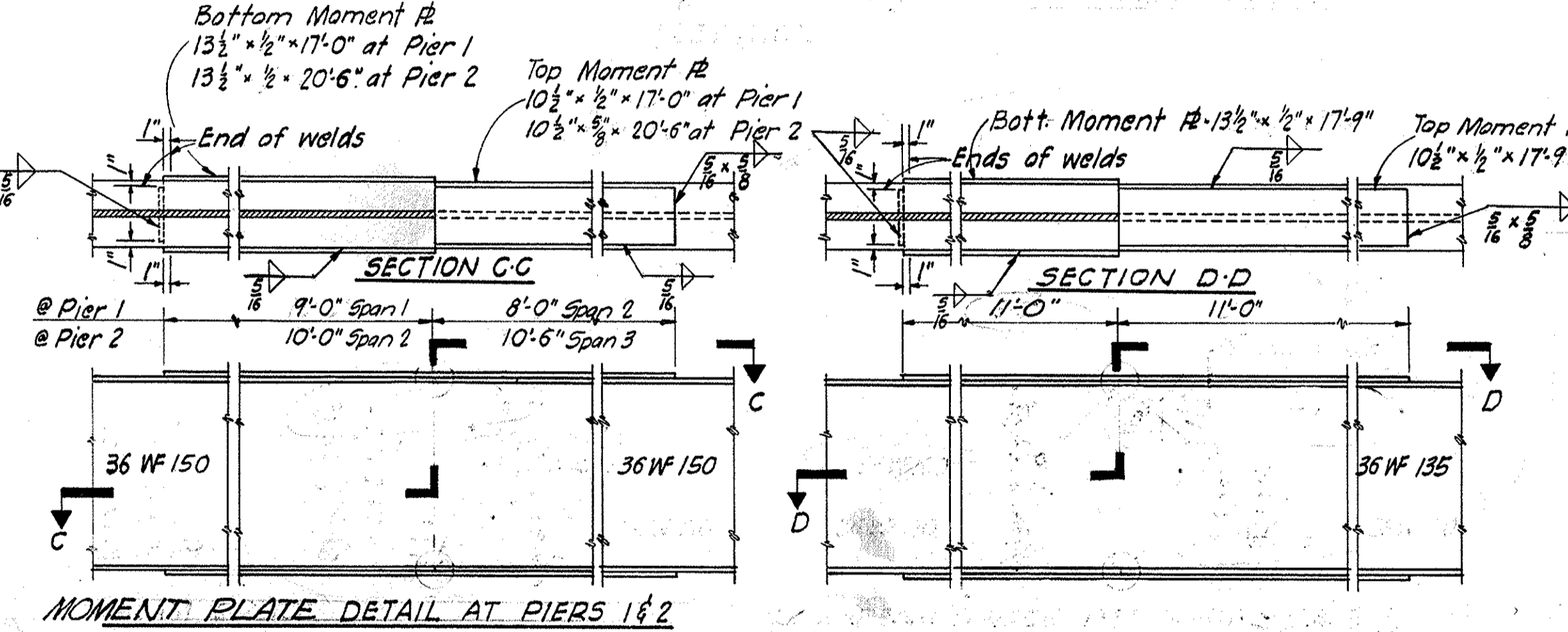


For Bolted Field Splice Details see Std. Dwg. 5D-2-G4.

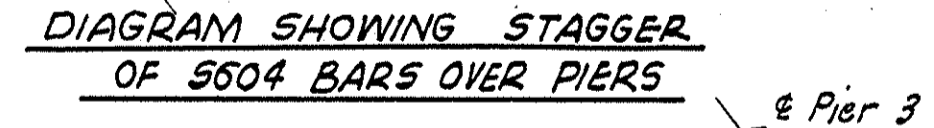
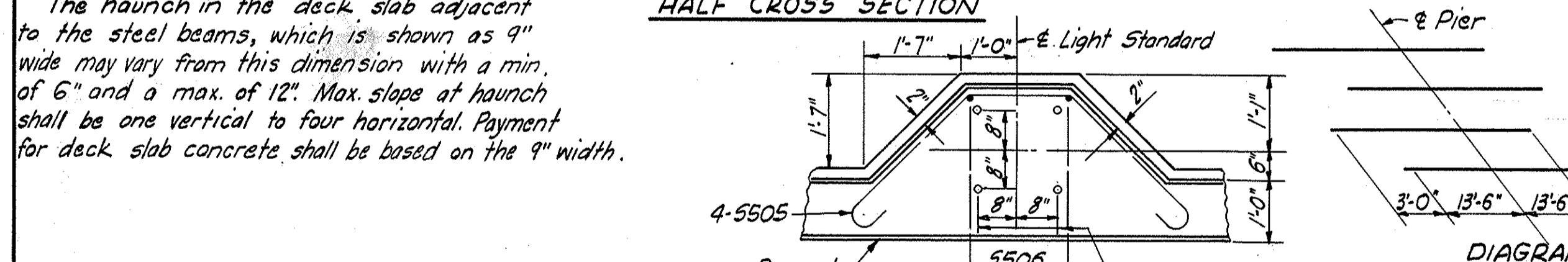
NOTES:

- Refer to Standard Drawing CSB-2-56, Sheets 2 1/3 of 6 for details of End Dams, End Crossframes, Curb Plates, 2" Pipe Drains, Scuppers, to Std. Dwg. AR-1-57 for details of Type "C" Aluminum Railing and Concrete Parapet, to Standard Drawing FSB-1-62 for details of Fixed & Sliding Bearings, and to 5D-2-G4 for Bolted Field Splice Details.

DECK SLAB HAUNCH
The haunch in the deck slab adjacent to the steel beams, which is shown as 9" wide may vary from this dimension with a min. of 6" and a max. of 12". Max. slope at haunch shall be one vertical to four horizontal. Payment for deck slab concrete shall be based on the 9" width.



- DECK CONSTRUCTION PROCEDURE:** In placing deck concrete, construction joints will be permitted parallel to the transverse reinforcing steel and near the middle of any span. Because of the flow of curing water from the surface of previously placed deck concrete, the sequence of pours shall be upgrade, starting at the low end.
- ELECTRICAL LIGHTING SYSTEM:** For additional details refer to drawings, "Details - Bridge Lighting and General Lighting Notes."

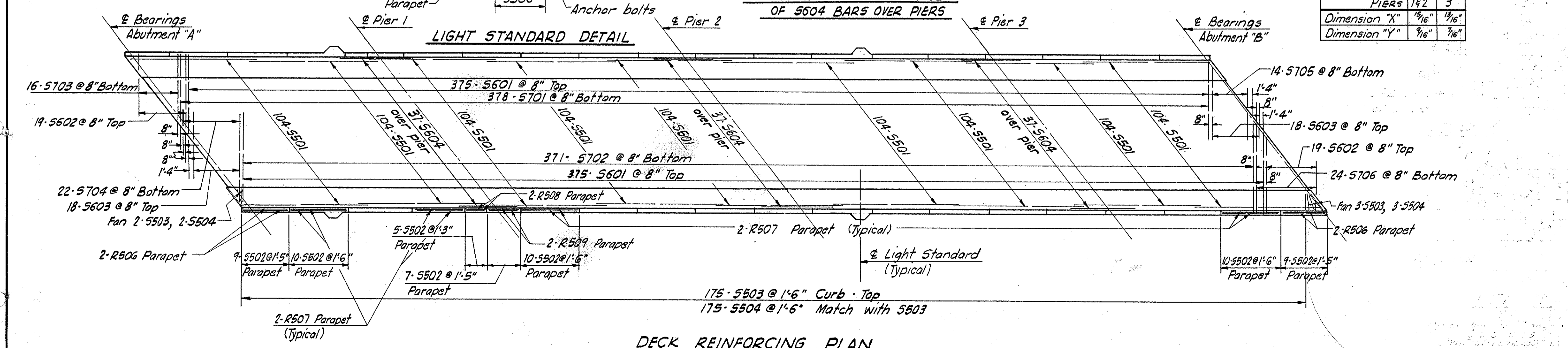


PIERS	1 & 2	3
Dimension "X"	9/16"	13/16"
Dimension "Y"	3/16"	7/16"

DEFLECTIONS & CAMBER

LOCATION	INTERIOR BEAMS				EXTERIOR BEAMS			
	SPAN 1	SPAN 2	SPAN 3	SPAN 4	SPAN 1	SPAN 2	SPAN 3	SPAN 4
Deflection Due to weight of steel	1/16"	1/16"	1/8"	0"	1/16"	1/16"	1/8"	0"
Deflection due to remaining D.L.	5/16"	3/8"	1/2"	5/8"	7/16"	9/16"	1 1/16"	3/8"
Convexity required for Vertical Curve	-5/8"	-1"	-1"	0"	-5/8"	-1"	-1"	0"
Sum of Deflection & Convexity	-1/4"	-3/16"	-3/8"	1/8"	-1/8"	-3/16"	-3/16"	3/16"
Camber required	0"	0"	0"	0"	0"	0"	0"	0"

NOTE: Place any natural camber down.



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ROCHESTER, PENNSYLVANIA

SUPERSTRUCTURE DETAILS
BRIDGE NO. MAH-18-1730
WELLINGTON AVENUE OVER
WICKLIFFE EXPRESSWAY

MAHONING COUNTY					STA. 613+36.06	
Designed	Drawn	Traced	Checked	Reviewed	Date	Revised
E.F.L.	M.A.M.	M.A.M.	R.L.S.	PK	8-16-13	

