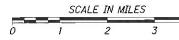
0

NONE

# LOCATION MAP

LATITUDE: 40°27'50" LONGITUDE: 83°04'32"





STRUCTURES:

FIANT

E-64877

DATE: 7/2/2020

ENGINEERS SEAL:

KATHERINE

MONTOYA

E-84480

SIGNED: Their Marting

Kern L. Frant

# DESIGN DESIGNATION

MAD-23 (1 56-2 30)

MAR-23 (1.30-2.33)	
CURRENT ADT (2018)	28,000
DESIGN YEAR ADT (2038)	37,000
DESIGN HOURLY VOLUME (2038)	3,700
DIRECTIONAL DISTRIBUTION	51%
TRUCKS (24 HOUR B&C)	10%
DESIGN SPEED	65
LEGAL SPEED	65
DESIGN FUNCTIONAL CLASSIFICATION:	
02 PRINCIPAL ARTERIAL (RURAL)	
NHS PROJECT	YES

# UNDERGROUND UTILITIES Contact Two Working Days Before You Dig OHIO811.org Before You Dig

OHIO811, 8-1-1, or 1-800-362-2764 (Non-members must be called directly)

PLAN PREPARED BY:



# STATE OF OHIO DEPARTMENT OF TRANSPORTATION

# MAR-23-1.85/2.04

# WALDO TOWNSHIP MARION COUNTY

# INDEX OF SHEETS:

TITLE		1
SCHEMATIC		2
TYPICAL SECTIONS	3	5
TYPICAL DETAILS		3
GENERAL NOTES	6 -	7
MAINTENANCE OF TRAFFIC	8 -	31
GENERAL SUMMARY	32 -	33
PAVEMENT SUBSUMMARY		34
ROADWAY SUBSUMMARY		35
TRAFFIC CONTROL SUBSUMMARY		36
PROJECT SITE PLAN		37
PLAN AND PROFILE	38 -	41
GUARDRAIL PLAN	42 -	53
TRAFFIC CONTROL PLAN	54 -	57
STRUCTURE OVER 20' SPAN		
MAR-23-0185	58 -	87
MAR-23-0204	88 -	112 & 90A

# PROJECT DESCRIPTION

REPLACE BRIDGE DECKS OF TWO TWIN STRUCTURES ON US-23 OVER SR-98 AND OU QUA CREEK IN MARION COUNTY NEAR THE TOWN OF WALDO. INCIDENTAL RESURFACING AND GUARDRAIL REPLACEMENTS.

# EARTH DISTURBED AREAS

PROJECT EARTH DISTURBED AREA:
ESTIMATED CONTRACTOR EARTH DISTURBED AREA:
NOTICE OF INTENT EARTH DISTURBED AREA:

3.47 ACRES 0.13 ACRES 4.90 ACRES 7)

E170(47

102332

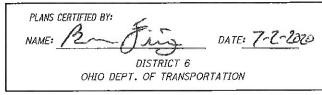
# LIMITED ACCESS

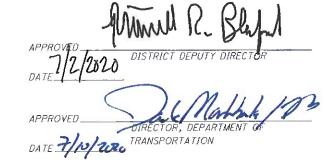
THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE OHIO REVISED CODE.

# 2019 SPECIFICATIONS

THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, INCLUDING SUPPLEMENTAL SPECIFICATIONS LISTED IN THE PLANS AND CHANGES LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY AND THAT PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE AS SET FORTH ON THE PLANS AND ESTIMATES.





DESIGN EXCEPTIONS

DESIGN FEATURE

SHOULDER WIDTH

APPROVAL DATE 02/15/18 SHEET NUMBERS 3 AND 103

		STANDA	ARD CONSTR	RUCTION DRA	WINGS				PLEMENTAL IFICATIONS
AS-1-15	07/17/15	MGS-1.1	01/19/18	MT-98.20	04/19/19	SICD-1-96	07/18/14	800	07/17/20
AS-2-15	01/19/18	MGS-2.1	01/19/18	MT-98.29	01/17/20	SICD-2-14	07/18/14	807	04/17/20
		MGS-3.1	01/19/18	MT-99.20	04/19/19			808	01/18/19
BP-3.1	01/17/20	MGS-3.2	01/18/13	MT-99.30	01/17/20	TC-41.20	10/18/13	821	04/20/12
BP5.1	01/18/19	MGS-4.2	07/19/13	MT-100.00	01/15/16	TC-42.20	10/18/13	832	10/19/18
BP-9.1	01/18/19	MGS-5.3	07/15/16	MT-101.60	01/17/20	TC-52.10	10/18/13	869	10/17/14
		MGS-6.1	01/19/18	MT-101.70	01/17/20	TC-52.20	01/19/18	875	01/18/19
DM-4.1	07/20/18			MT-101.75	01/17/20	TC-61.10	01/17/20	878	01/17/20
DM-4.3	01/15/16	MT-95.30	07/19/19	MT-101.80	01/17/20	TC-61.30	07/19/19	896	07/21/17
DM-4.4	01/15/16	MT-95.40	01/17/20	MT-101.90	07/21/17	TC-65.10	01/17/14		
		MT-95.50	07/21/17	MT-104.10	10/16/15	TC-65.11	07/21/17		
PCB-91	01/18/13	MT-95.70	01/17/20	MT-105.10	01/17/20	TC-72.20	07/15/16	S	PECIAL
		MT-95.82	07/19/13					PRO	OVISIONS
RM-4.2	04/17/20	MT-97.10	04/19/19	SBR-1-13	01/17/14	VPF-1-90	07/20/18		

1 112

Σ

S

Z

⋖

Z

Ш

 $\vdash$ 

AIN

Σ

2

# MAINTENANCE OF TRAFFIC - SEQUENCE OF OPERATIONS:

PRE-PHASE 1 - WORK IN THE PRE-PHASE SHALL INCLUDE THE CONSTRUCTION OF THE MEDIAN CROSSOVERS AND TEMPORARY PAVEMENT.

PHASE 1 - WORK IN THIS PHASE SHALL INCLUDE REHABILITATION OF NORTHBOUND STRUCTURES MAR-23-0185R AND MAR-23-0204R AND THE NORTHBOUND LANES OF US-23.

PHASE 2 - WORK IN THIS PHASE SHALL INCLUDE REHABILITATION OF SOUTHBOUND STRUCTURES MAR-23-0185L AND MAR-23-0204L AND THE SOUTHBOUND LANES OF US-23.

POST-PHASE 2 - WORK IN THE POST-PHASE A SHALL INCLUDE THE REMOVAL OF THE CROSSOVERS, INSTALLATION OF FINAL STRIPING, AND BRIDGE PAINTING. IT MAY ALSO INCLUDE THE INSTALLATION OF GUARDRAIL NOT CONNECTED TO THE STRUCTURES, CONCRETE SEALING, AND/OR PAVEMENT REPAIRS NOT COMPLETED DURING PRE-PHASE 1, PHASE 1, OR PHASE 2.

# MAINTENANCE OF TRAFFIC:

BELOW IS A SUMMARY OF MOT REQUIREMENTS FOR THIS PROJECT. THE CONTRACTOR SHALL NOT ENTER PHASE I UNTIL APRIL 1, 2021 UNLESS APPROVED BY THE ENGINEER.

# PRE-PHASE 1:

\* UTILIZE SHOULDER AND LANE CLOSURES.

### PHASE 1:

 $\bigcirc$ 

\* REDUCE BOTH SOUTHBOUND AND NORTHBOUND TRAFFIC FROM 2 LANES TO 1 LANE. CROSSOVER THE SINGLE NORTHBOUND LANE TO THE SOUTHBOUND SIDE OF US-23, CREATING TWO-LANE, TWO-WAY TRAFFIC. CROSSOVER THE SINGLE NORTHBOUND LANE BACK TO THE NORTHBOUND SIDE AFTER TRAFFIC HAS PASSED THE NORTHERNMOST WORK ZONE. ALL RAMPS SHALL REMAIN OPEN.

### PHASE 2:

\* REDUCE BOTH SOUTHBOUND AND NORTHBOUND TRAFFIC FROM 2 LANES TO 1 LANE. CROSSOVER THE SINGLE SOUTHBOUND LANE TO THE NORTHBOUND SIDE OF US-23, CREATING TWO-LANE, TWO-WAY TRAFFIC. CROSSOVER THE SINGLE SOUTHBOUND LANE BACK TO THE SOUTHBOUND SIDE AFTER TRAFFIC HAS PASSED THE SOUTHERNMOST WORK ZONE. RAMP A, THE EXIT RAMP FROM US-23 SOUTHBOUND TO SR-98, SHALL BE CLOSED. ALL OTHER RAMPS SHALL REMAIN OPEN.

# POST-PHASE 2

\* UTILIZE SHOULDER AND LANE CLOSURES.

SINGLE LANE CLOSURES AS PER MT-95.30 MAY BE UTILIZED WHEN THE CROSSOVERS ARE NOT IN PLACE. CLOSURES SHALL FOLLOW THE LANE VALUE CONTRACT TABLE ON SHEET 13.

SINGLE LANE CLOSURES USING FLAGGERS AS PER MT-97.10 SHALL BE UTILIZED ON SR-98.

## TRUCK MOUNTED ATTENUATOR (TMA):

WHEN WORKING IN A CLOSED LANE OR SHOULDER ON A MULTILANE HIGHWAY WITHOUT TEMPORARY OR PERMANENT TRAFFIC BARRIERS SEPARATING THE WORK AREA FROM THEM TRAVELED LANES, A TRUCK MOUNTED ATTENUATOR (TMA) SHALL BE PROVIDED TO PROTECT EACH WORK AREA IN ACCORDANCE WITH STANDARD DRAWINGS MT-95.30, MT-95.31, MT-95.32 OR OMUTCD TYPICAL APPLICATION (TA) 4 AND TA-6. THE TMA SHALL BE PLACED IN SUCH A WAY TO ADEQUATELY PROTECT THE WORKERS INSIDE THE WORK ZONE. THE TMA IS NOT INTENDED TO BE USED AS OR SUBSTITUTED FOR THE FLASHING ARROW PANEL AT THE BEGINNING OF THE MERGE TAPER. THE TMA SHALL MEET NCHRP 350 TEST LEVEL 3 CRITERIA FOR STANDARD AND OPTIONAL TESTS AT 100 KM/H (62 MPH) FOR DESIGN IMPACTS. THE COST FOR PROVIDING THEM TMA SHALL INCLUDE ALL MATERIAL, LABOR, EQUIPMENT, AND HARDWARE REPLACEMENT AND IS TO BE INCLUDED IN THE LUMP SUM BID PRICE FOR ITEM 614 - MAINTAINING TRAFFIC.

# RIGHT OF WAY PERMITS:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL APPLICABLE RIGHT OF WAY USE PERMITS TO INSTALL MAINTENANCE OF TRAFFIC SIGNING.

# ITEM 614 - LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE DURING CONSTRUCTION OPERATIONS:

USE OF LAW ENFORCEMENT OFFICERS (LEOS) BY CONTRACTORS OTHER THAN THE USES SPECIFIED BELOW WILL NOT BE PERMITTED AT PROJECT COST. LEOS SHOULD NOT BE USED WHERE THE OMUTCD INTENDS THAT FLAGGERS BE USED.

IN ADDITION TO THE REQUIREMENTS OF C&MS 614 AND THE OMUTCD, A UNIFORMED LEO WITH AN OFFICIAL PATROL CAR (CAR WITH TOP-MOUNTED EMERGENCY FLASHING LIGHTS AND COMPLETE MARKINGS OF THE APPROPRIATE LAW ENFORCEMENT AGENCY) SHALL BE PROVIDED FOR THE FOLLOWING TRAFFIC CONTROL TASKS:

DURING THE ENTIRE ADVANCE PREPARATION AND CLOSURE SEQUENCE WHERE COMPLETE BLOCKAGE OF TRAFFIC IS REQUIRED.

IN ADDITION TO THE REQUIREMENTS OF C&MS 614 AND THE OMUTCD, A
UNIFORMED LEO WITH AN OFFICIAL PATROL CAR (CAR WITH TOP-MOUNTED
EMERGENCY FLASHING LIGHTS AND COMPLETE MARKINGS OF THE
APPROPRIATE LAW ENFORCEMENT AGENCY) SHOULD BE PROVIDED FOR THE
FOLLOWING TRAFFIC CONTROL TASKS AS APPROVED BY THE ENGINEER:

\* FOR LANE CLOSURES: DURING INITIAL SET-UP PERIODS, TEAR DOWN PERIODS, SUBSTANTIAL SHIFTS OF A CLOSURE POINT OR WHEN NEW LANE CLOSURE ARRANGEMENTS ARE INITIATED FOR LONG-TERM LANE CLOSURES/SHIFTS (FOR THE FIRST AND LAST DAY OF MAJOR CHANGES IN TRAFFIC CONTROL SETUP). IN GENERAL, LEOS SHOULD BE POSITIONED AT THE POINT OF LANE RESTRICTION OR ROAD CLOSURE AND TO MANUALLY CONTROL TRAFFIC MOVEMENTS THROUGH INTERSECTIONS IN WORK ZONES.

IN GENERAL, LEOS SHOULD BE POSITIONED IN ADVANCE OF AND ON THE SAME SIDE AS THE LANE RESTRICTION OR AT THE POINT OF ROAD CLOSURE, AND TO MANUALLY CONTROL TRAFFIC MOVEMENTS THROUGH SIGNALIZED INTERSECTIONS IN WORK ZONES.

LEOS SHOULD NOT FORGO THEIR TRAFFIC CONTROL RESPONSIBILITIES TO APPREHEND MOTORISTS FOR ROUTINE TRAFFIC VIOLATIONS. HOWEVER, IF A MOTORIST'S ACTIONS ARE CONSIDERED TO BE RECKLESS, THEN PURSUIT OF THE MOTORIST IS APPROPRIATE.

THE LEOS WORK AT THE DIRECTION OF THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR SECURING THE SERVICES OF THE LEOS WITH THE APPROPRIATE AGENCIES AND COMMUNICATING THE INTENTIONS OF THE PLANS WITH RESPECT TO DUTIES OF THE LEOS. THE ENGINEER SHALL HAVE FINAL CONTROL OVER THE LEO'S DUTIES AND PLACEMENT, AND WILL RESOLVE AND ISSUES THAT MAY ARISE BETWEEN THE TWO PARTIES

ENSURE PROVIDED LEOS HAVE BEEN TRAINED APPROPRIATE TO THE JOB DECISIONS THEY ARE REQUIRED TO MAKE WHILE ON THE PROJECT, IN ACCORDANCE WITH C&MS 614.03.

THE LEO SHALL REPORT IN TO THE CONTRACTOR PRIOR TO THE START OF THE SHIFT, IN ORDER TO RECEIVE INSTRUCTIONS REGARDING SPECIFIC WORK ASSIGNMENTS DURING HIS/HER SHIFT. THE LEO IS EXPECTED TO STAY AT THE PROJECT SITE FOR THE ENTIRE DURATION OF HIS/HER SHIFT. THE LEO SHALL REPORT TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT. ONCE THE LEO HAS COMPLETED THE DUTIES DESCRIBED ABOVE AND STILL HAS TIME REMAINING ON HIS/HER SHIFT, THE LEO MAY BE ASKED TO PATROL THROUGH THE WORK ZONE (WITH FLASHING LIGHTS OFF) OR BE PLACED AT A LOCATION TO DETER MOTORISTS FROM SPEEDING. SHOULD IT BE NECESSARY TO LEAVE THE PROJECT SITE, THE LEO SHALL NOTIFY THE ENGINEER. THE CONTRACTOR SHALL PROVIDE THE LEO WITH A TWO-WAY COMMUNICATION DEVICE WITH SHALL BE RETURNED TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT.

LEOS (WITH PATROL CAR) REQUIRED BY THE TRAFFIC MAINTENANCE TASKS ABOVE SHALL BE PAID FOR ON A UNIT PRICE (HOURLY) BASIS UNDER ITEM 614, LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE. THE HOURS PAID SHALL INCLUDE ANY MINIMUM SHOW-UP TIME REQUIRED BY THE LAW ENFORCEMENT AGENCY INVOLVED. ANY ADDITIONAL COSTS

(ADMINISTRATIVE OR OTHERWISE) INCURRED BY THE CONTRACTOR TO OBTAIN THE SERVICES OF AN LEO ARE INCLUDED WITH THE BID UNIT PRICE FOR ITEM, 614 LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE.

THE FOLLOWING ESTIMATED QUANTITY HAS BEEN PROVIDED AND CARRIED TO THE GENERAL SUMMARY:

ITEM 614 - LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE

= 100 HOUR

# APPROVED MAINTENANCE OF TRAFFIC (MOT) POLICY EXCEPTION:

PORTIONS OF THE MOT PLANS AS DESCRIBED BELOW HAVE BEEN APPROVED BY THE MOT EXCEPTION COMMITTEE (MOTEC) OR THE PROJECT IMPACT ADVISORY COUNCIL (PIAC) PER TRAFFIC MANAGEMENT IN WORK ZONES POLICY (21-008(P)) AND STANDARD PROCEDURE (123-001(SP)).

APPROVED MOT EXCEPTIONS INCLUDE:
REDUCE US 23 IN BOTH DIRECTIONS FROM 2 TO 1 LANE VIA CROSSOVER,
APPROVED 8/11/17.

A MAINTENANCE OF TRAFFIC MEETING SHALL BE HELD A MINIMUM OF 30 CALENDAR DAYS PRIOR TO IMPLEMENTATION OF EACH APPROVED MOT EXCEPTION. THIS MEETING SHALL INCLUDE THE DISTRICT WORK ZONE TRAFFIC MANAGER AND MARION COUNTY ENGINEER AS WELL AS THE CONTRACTOR, WORKSITE TRAFFIC SUPERVISOR, AND ANY SUBCONTRACTORS INVOLVED WITH TEMPORARY TRAFFIC CONTROL.

IN ADDITION TO ANY NOTIFICATIONS REQUIRED IN OTHER NOTES, THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER AT LEAST 3 BUSINESS DAYS IN ADVANCE OF IMPLEMENTATION OF THE APPROVED MOT EXCEPTION REFERENCED ABOVE SO THAT THE PROJECT ENGINEER CAN SEND EMAIL NOTIFICATION TO THE OFFICE OF ROADWAY ENGINEERING, STATEWIDE TMC, DWZTM, AND SPECIAL HAULING PERMITS AT LEAST 2 BUSINESS DAYS IN ADVANCE OF THE IMPLEMENTATION OF THE APPROVED MOT EXCEPTION REFERENCED ABOVE. REFERENCE "EXCEPTION REQUEST APPROVAL DATED 8/11/17 FOR PID 102332" IN THE NOTIFICATION AND OTHER CORRESPONDENCE.

ANY CHANGES TO THE MOT THAT IMPACT THE PREVIOUSLY APPROVED MOT EXCEPTION LISTED ABOVE SHALL BE APPROVED IN WRITING BY THE APPLICABLE ODOT CENTRAL OFFICE COMMITTEE (MOTEC OR PIAC). IN THE EVENT THAT SUCH CHANGES ARE PROPOSED, THE REQUEST SHALL BE COORDINATED THROUGH THE DISTRICT WORK ZONE TRAFFIC MANAGER (DWZTM) A MINIMUM OF 30 CALENDAR DAYS PRIOR TO THE DESIRED IMPLEMENTATION DATE. IF THE DISTRICT AGREES WITH THE PROPOSED CHANGES THE DWZTM SHALL SEEK APPROVAL FROM THE APPLICABLE ODOT CENTRAL OFFICE COMMITTEE. IN THE EVENT THE PROPOSED CHANGES ARE APPROVED IN WRITING, THE CLOSURES ARE STILL SUBJECT TO NOTIFICATION REQUIREMENTS WITHIN THIS NOTE PRIOR TO IMPLEMENTATION.

# USE OF STANDARD DRAWINGS:

FOR THE PURPOSE OF THIS PROJECT, "MOVING OPERATION" SHALL BE LIMITED TO PAVEMENT MARKING STRIPING. IT MAY BE NECESSARY TO EXTEND THE ADVANCE WARNING AND BUFFER ZONES BEYOND THE MINIMUM DISTANCES SHOWN ON THE STANDARD DRAWINGS. THIS MAY BE DUE TO HORIZONTAL ALIGNMENT, VERTICAL ALIGNMENT, RAMP LOCATIONS, OR OTHER SIGHT OBSTRUCTIONS. LOCATIONS OF THE TAPER ZONES MAY BE ADJUSTED AS DIRECTED BY THE ENGINEER, BUT TAPER LENGTHS MUST MEET THE MINIMUM STANDARDS. TAPERS SHOULD BE PLACED IN TANGENT SECTIONS WHENEVER POSSIBLE. ADDITIONAL YIELD SIGNS MAY BE REQUIRED FOR RAMPS WITHIN 1,000 FEET OF A WORK ZONE. PAYMENT SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC.

FOR ANY MULTILANE HIGHWAY, DEVICE SPACING SHALL BE A MAXIMUM OF 40' (FEET) CENTER ON CENTER IN THE TAPERS AND 80' (FEET) CENTER ON CENTER IN THE TANGENT SECTIONS.



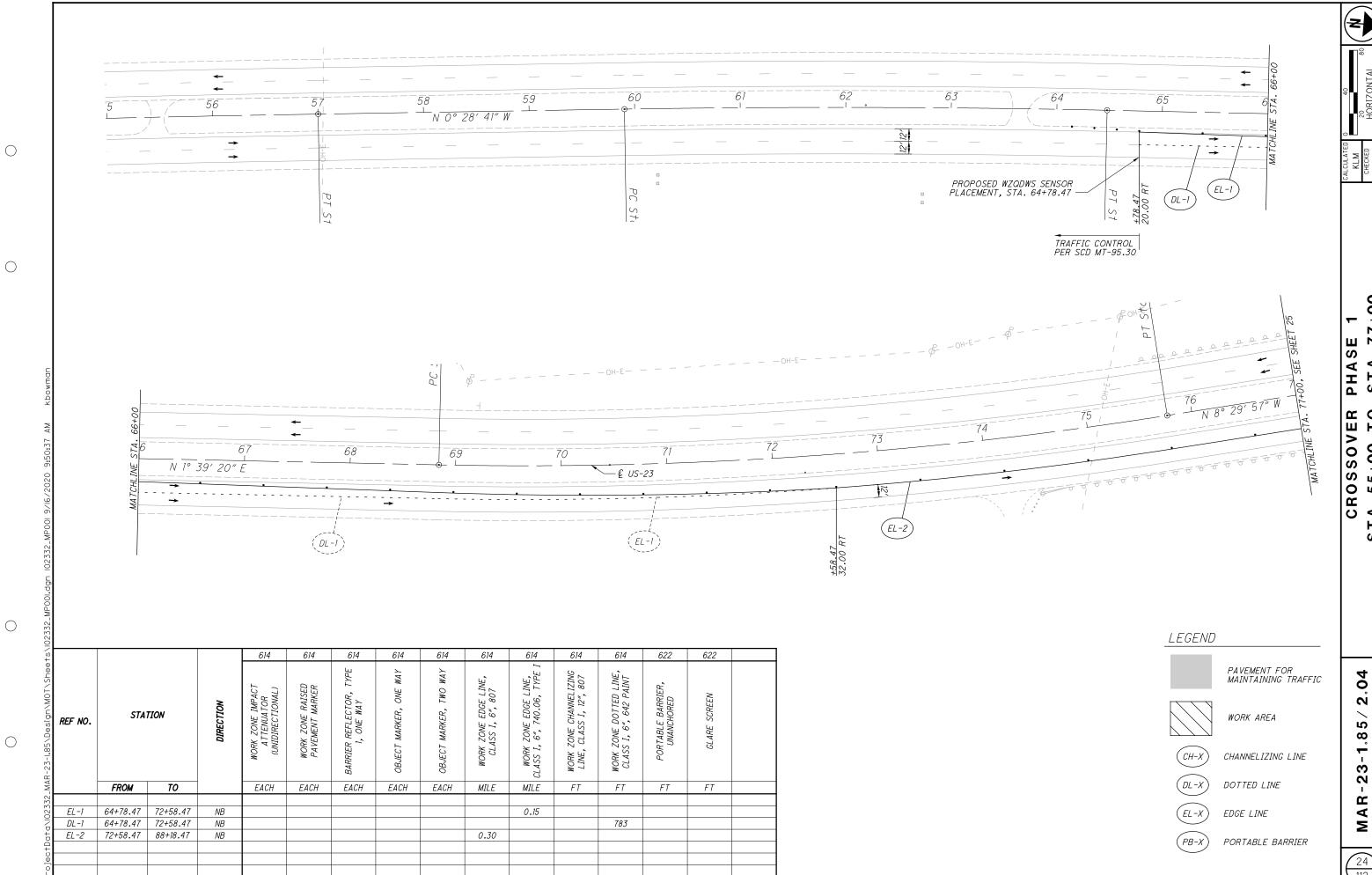
0
•
2
\
5
ω
_
ı
က
2
ı
$\mathbf{\alpha}$
⋖
5

				202	411	614	614	614	614	614	614	614	614	614	615	622	622	
SHEET	STA	TION	PHASE	PAVEMENT REMOVED, ASPHALT	STABILIZED CRUSHED AGGREGATE	WORK ZONE IMPACT ATTENUATOR (UNIDIRECTIONAL)	WORK ZONE RAISED PAVEMENT MARKER	BARRIER REFLECTOR, TYPE 1, ONE WAY	OBJECT MARKER, ONE WAY	OBJECT MARKER, TWO WAY	WORK ZONE EDGE LINE, CLASS I, 6", 807	WORK ZONE EDGE LINE, CLASS I, 6", 740.06, TYPE I	WORK ZONE CHANNELIZING LINE, CLASS I, 12", 807	WORK ZONE DOTTED LINE, CLASS I, 6", 642 PAINT	PAVEMENT FOR MAINTAINING TRAFFIC, CLASS A	PORTABLE BARRIER, UNANCHORED	GLARE SCREEN	
	FROM	то		SY	CY	EACH	EACH	EACH	EACH	EACH	MILE	MILE	FT	FT	SY	FT	FT	
18	55+00.00	154+00.00		3741	51										5234			
19	55+00.00	154+00.00		2941	54										4363			
24	66+00.00	88+00.00	1								0.30	0.15		783				
25	88+00.00	110+00.00	1			2		52	5	45	1.28	0.55				2500	2500	
26	110+00.00	132+00.00	1			1	26	7	6		0.46	0.12	470	563		310	310	
27	132+00.00	154+00.00	1								0.07	0.21		779				
28	55+00.00	77+00.00	2									0.15		784				
29	77+00.00	99+00.00	2			1	21	19		17	1.97	0.18	392	416		820	820	
30	99+00.00	121+00.00	2			2		42	12	28	0.37	0.25				1960	1960	
31	121+00.00	143+00.00	2									0.15		778				

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 



0.30

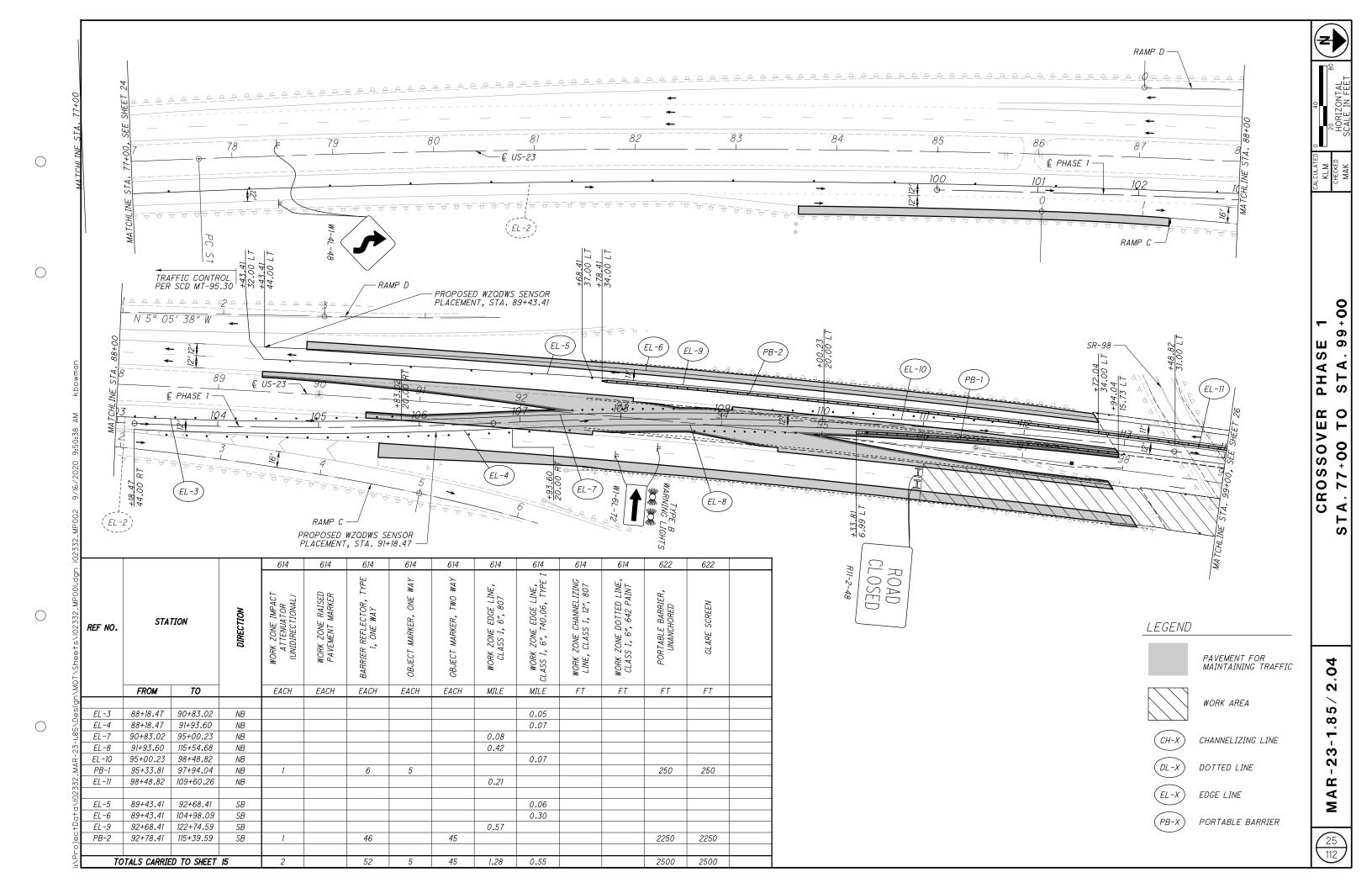
783

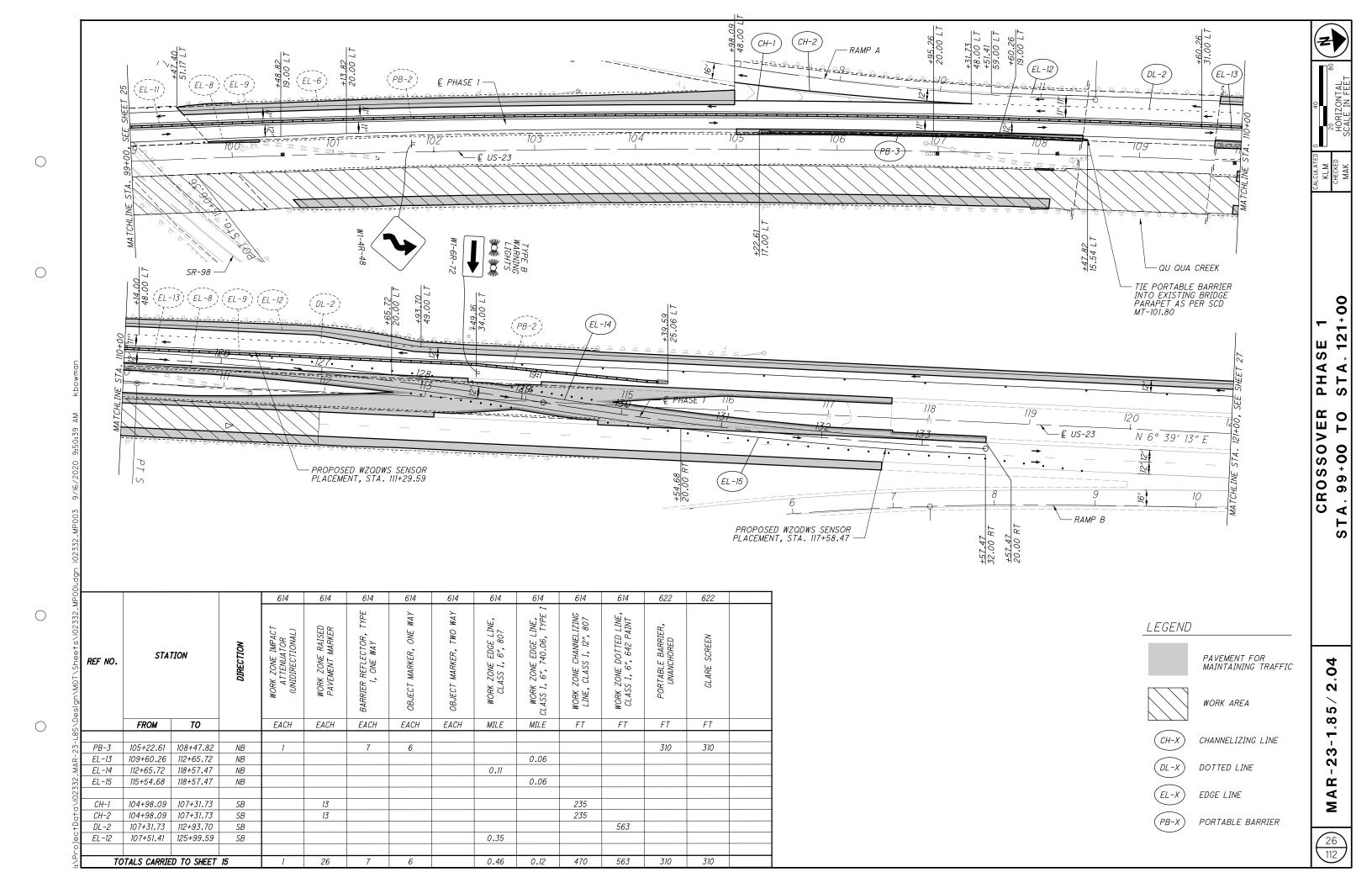
TOTALS CARRIED TO SHEET 15

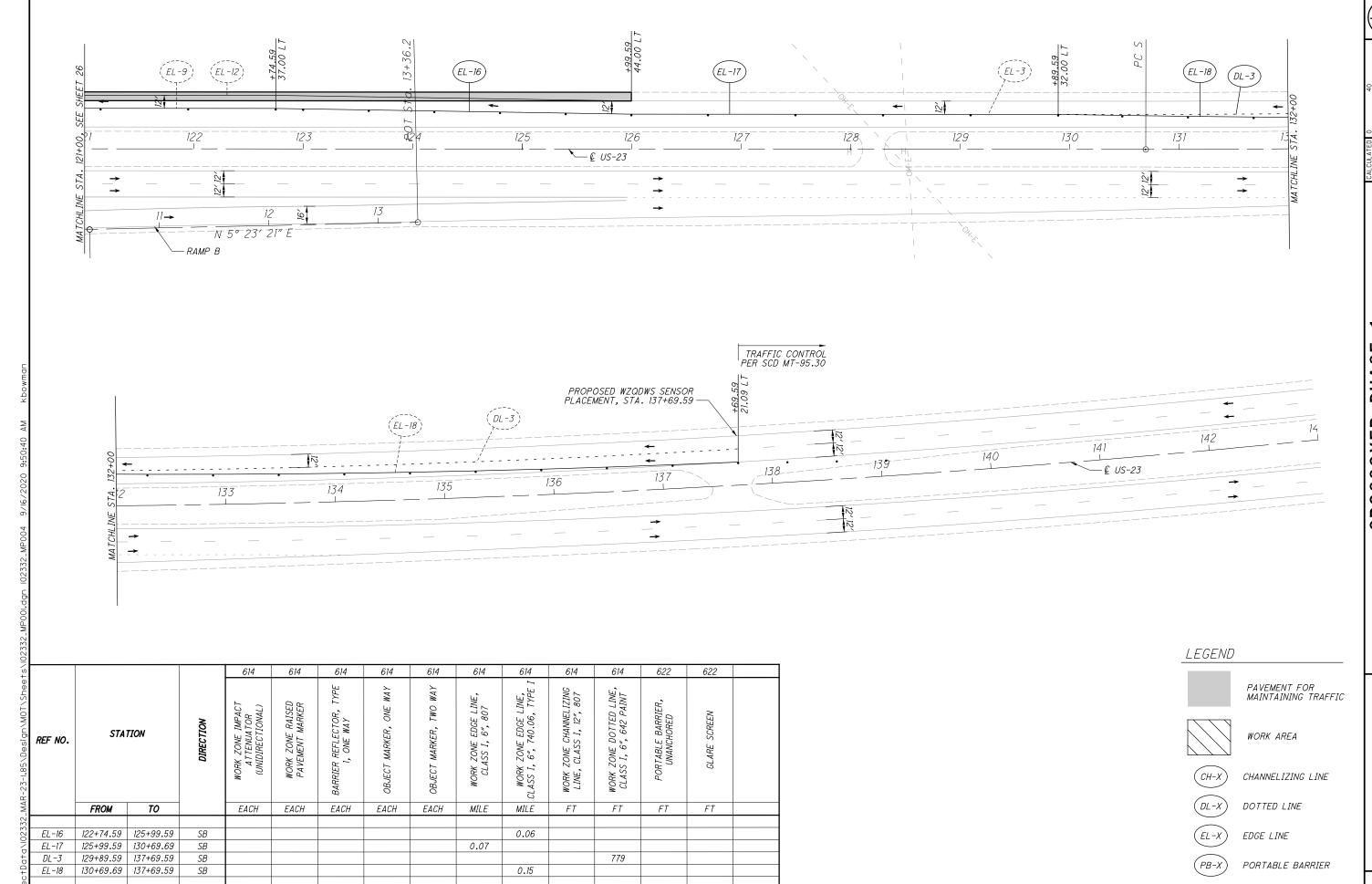
**1** + **00** PHASE STA, 77

CROSSOVER FA. 55+00 TO









0.07

0.21

779

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

TOTALS CARRIED TO SHEET 15

ASE 1 A,143+00

CROSSOVER PHASE 1 STA.121+00 TO STA.143

2.04

23-1.85

27

MAR



LEGEND	
	PAVEMENT FO

PAVEMENT FOR MAINTAINING TRAFFIC



WORK AREA



CHANNELIZING LINE

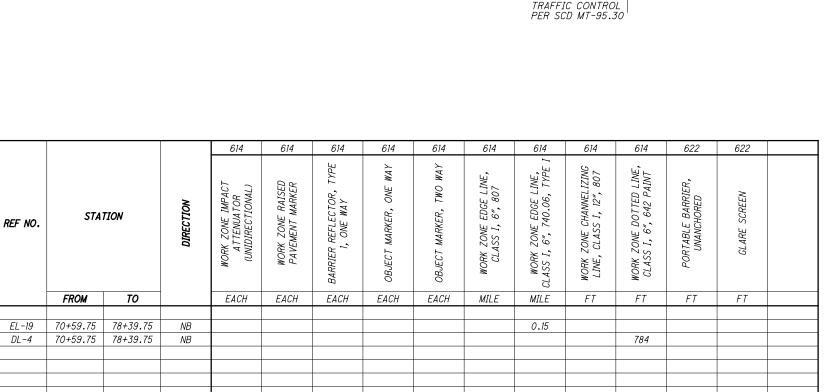


DOTTED LINE



) EDGE LINE





784

1,dgn 102332\_MP101 9/16/2

TOTALS CARRIED TO SHEET 15

 $\circ$ 

 $\bigcirc$ 

 $\bigcirc$ 

 $\circ$ 

28

**6** 

2

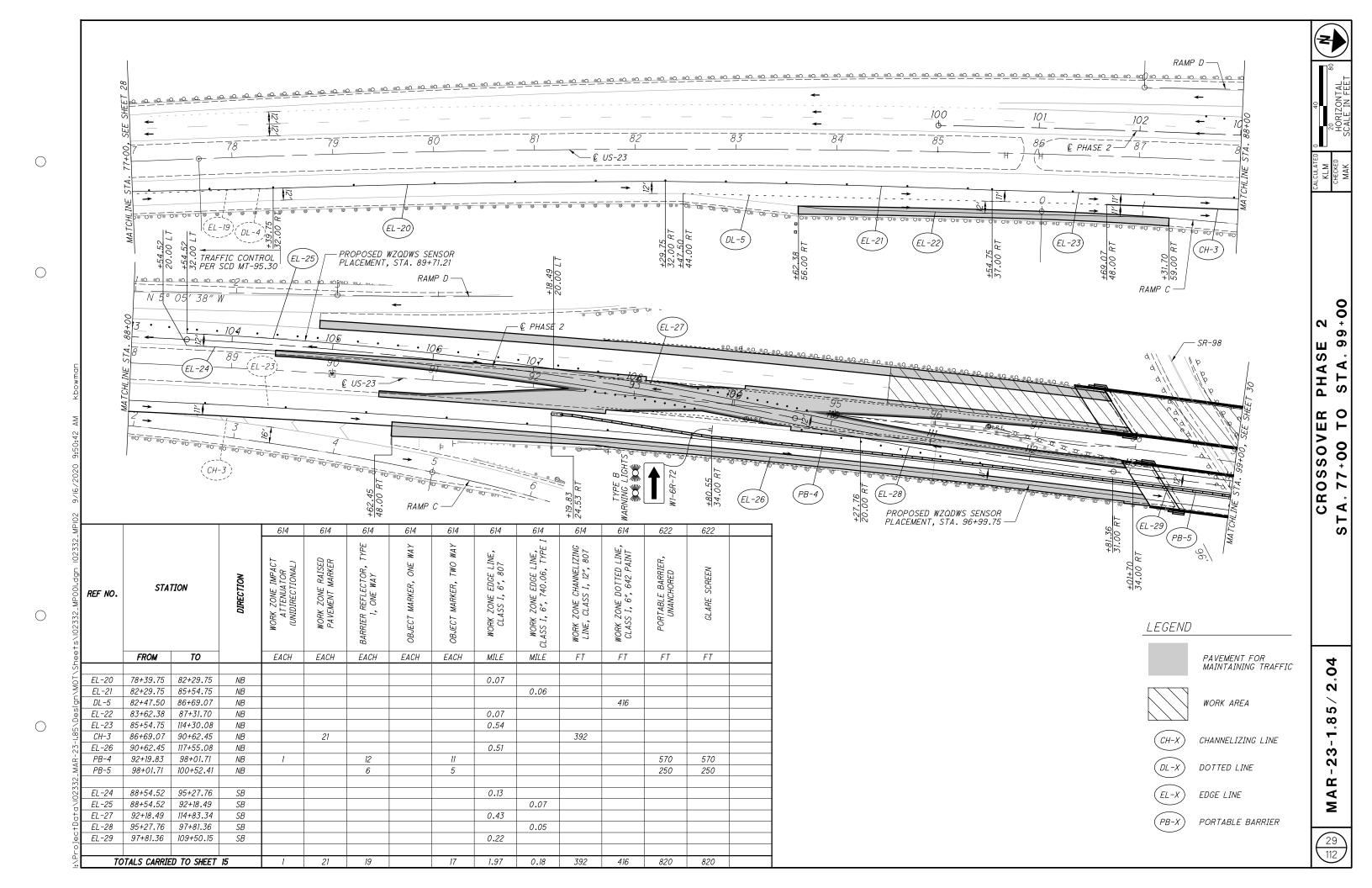
5/

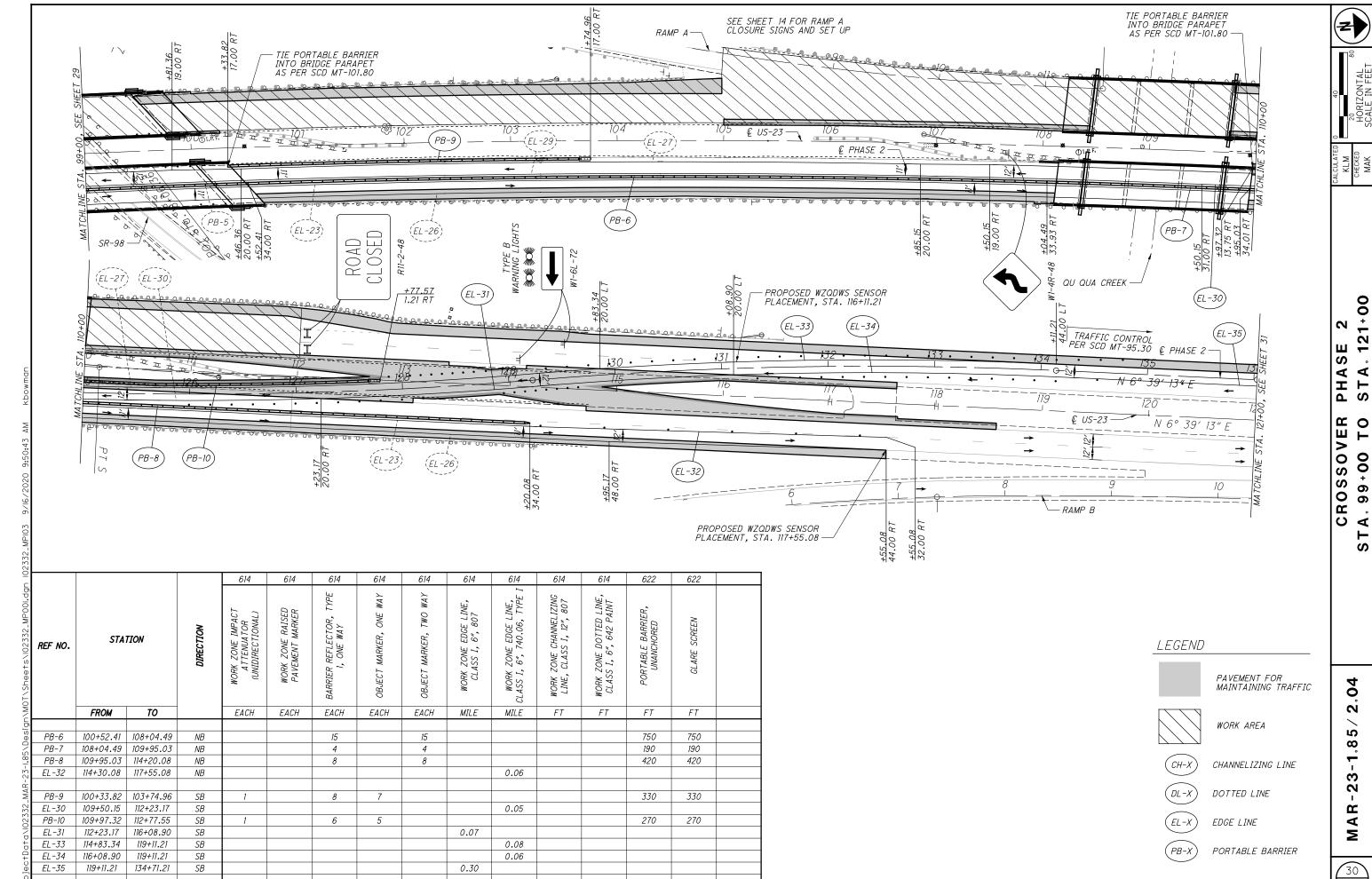
ထ

Ţ

23

MAR-





1960

1960

 $\bigcirc$ 

 $\bigcirc$ 

TOTALS CARRIED TO SHEET 15

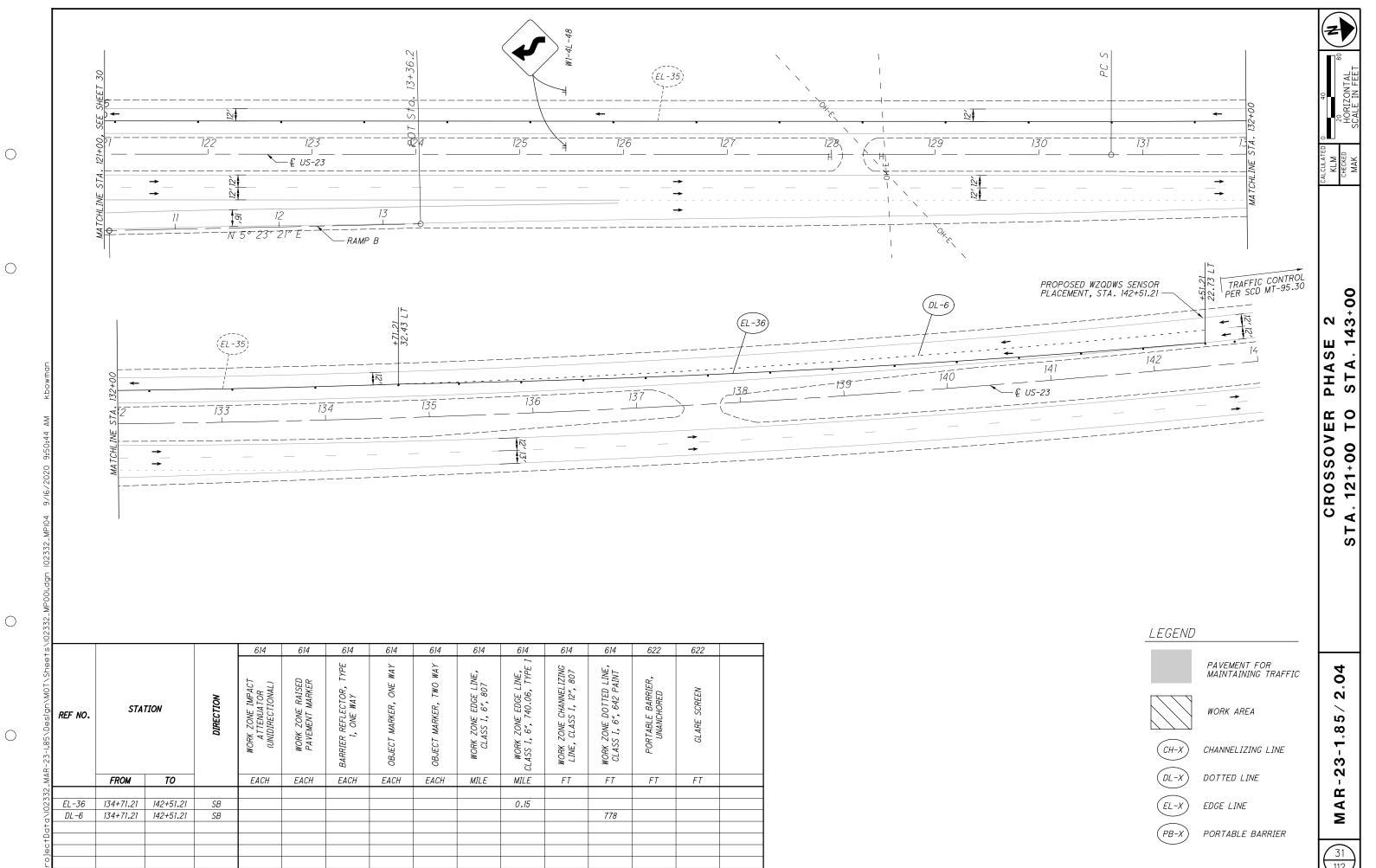
42

0.37

0.25

9 3 ⋖ PH/ ST

112



0.15

778

TOTALS CARRIED TO SHEET 15

00

STA CROSSOVER A. 121+00 TO

2

				OUEET AND SEE		1,12	RTICIPATION	<b></b>		05	,		SE SHE	
				SHEET NUMBER		01/NHS/BR 02/SAF/OT		ITEM	EXT	GRAND TOTAL	UNIT	DESCRIPTION	NO.	
6-7	8-13	15	34	35 36										
												TRAFFIC CONTROL (CONT.)		
				1.81		1.81		644	00104	1.81	MILE	EDGE LINE, 6"		
				1.74		1.74		644	00204	1.74	MILE	LANE LINE, 6"		
				896 135		896 135		644 644	00404 00720	896 135	FT FT	CHANNELIZING LINE, 12"  CHEVRON MARKING		
				881		881		644	01510	881	FT	DOTTED LINE, 6"		
				0.32		0.32		646	10010	0.32	MILE	EDGE LINE, 6"		
				0.16		0.16 181		646 646	10110 20504	0.16 181	MILE FT	LANE LINE, 6"  DOTTED LINE, 6"		
				101		101		0.10	20001	101	, ,	borres ener, o		
												STRUCTURE OVER 20 FOOT SPAN (SFN 5100305 & 5100364)  SEE SHEET 62		
												STRUCTURE OVER 20 FOOT SPAN (SFN 5100399 & 5100429)		
												SEE SHEET 91		
	100					100		614	11110	100	HOUR	MAINTENANCE OF TRAFFIC  LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE		
	100	6				6		614	11110 12336	6	HOUR EACH	WORK ZONE IMPACT ATTENUATOR (UNIDIRECTIONAL)		
	LS					LS		614	12421	LS		DETOUR SIGNING, AS PER PLAN		
	4					4		614	12484	4	EACH	WORK ZONE INCREASED PENALTIES SIGN		
	2					2		614	12756	2	EACH	WORK ZONE CROSSOVER LIGHTING SYSTEM		
		47				47		614	12800	47	EACH	WORK ZONE RAISED PAVEMENT MARKER		
		120				120		614	13310	120	EACH	BARRIER REFLECTOR, TYPE 1, ONE-WAY		
		23				23		614	13350	23	EACH	OBJECT MARKER, ONE WAY		
	15	90				90 15		614 614	13360 18601	90 15	EACH SNMT	OBJECT MARKER, TWO WAY  PORTABLE CHANGEABLE MESSAGE SIGN, AS PER PLAN		
	10					10		014	10001	10	SIVIVI	TOTTABLE CHANGLABLE MESSAGE STORY, AS TENTERN		
	2.03					2.03		614	20560	2.03	MILE	WORK ZONE LANE LINE, CLASS III, 6", 642 PAINT		
		4.45 1.76				1.76	4.45	614 614	22056	4.45	MILE	WORK ZONE EDGE LINE, CLASS I, 6", 807  WORK ZONE EDGE LINE, CLASS I, 6", 740.06, TYPE I		
	1.60	1.10				1.76 1.60		614	22210 22360	1.76 1.60	MILE MILE	WORK ZONE EDGE LINE, CLASS II, 6", 140.00, TITE I		
		862				,,,,,	862	614	23110	862	FT	WORK ZONE CHANNELIZING LINE, CLASS I, 12", 807		
	070					070		-	07000	070		WORK TOUS SHAWE ITTUS AND SHEET OF SOLUTIONS OF SHEET		
	279	4103				279 4103		614 614	23690 24202	279 4103	FT FT	WORK ZONE CHANNELIZING LINE, CLASS III, 12", 642 PAINT  WORK ZONE DOTTED LINE, CLASS I, 6", 642 PAINT		
	554	4103				554		614	24612	554	FT	WORK ZONE DOTTED LINE, CLASS III, 6", 642 PAINT		
						LS		615	10000	LS		ROADS FOR MAINTAINING TRAFFIC		
		9597				9597		615	20000	9597	SY	PAVEMENT FOR MAINTAINING TRAFFIC, CLASS A		
		5590				5590		622	41100	5590	FT	PORTABLE BARRIER, UNANCHORED		
		5590				5590		622	80000	5590	FT	GLARE SCREEN		
	12					12		808	18700	12	SNMT	DIGITAL SPEED LIMIT (DSL) SIGN ASSEMBLY		
	36 12					36 12		896 896	00010 00021	36 12	SNMT SNMT	PORTABLE NON-INTRUSIVE TRAFFIC SENSOR, CLASS I  PORTABLE CHANGEABLE MESSAGE SIGN. AS PER PLAN		
	12					12		030	00021	12	SIVIVII	TONTABLE CHANGLABLE MESSAGE SIGN, AS TENTEAN		
												INCIDENTALS		
						LS		108	30000	LS		CPM PROGRESS SCHEDULE SHORT DURATION PROJECTS		
						LS 7		614 619	11000 16010	<u>LS</u> 7	MNTH	MAINTAINING TRAFFIC FIELD OFFICE, TYPE B		
						LS		623	10001	LS	IVIIV I FI	CONSTRUCTION LAYOUT STAKES AND SURVEYING, AS PER PLAN		
						LS		624	10000	LS		MOBILIZATION		

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATION(S): SS869 DATED/REVISED 10-17-14

# DESIGN SPECIFICATIONS

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND STRANSPORTATION OFFICIALS, 17TH EDITION, 2002 AND THE ODOT BRIDGE DESIGN MÁNUAL, 2004.

### DESIGN LOADING

 $\bigcirc$ 

HS-20-44 CASE II AND THE ALTERNATE MILITARY LOADING (SUPERSTRUCTURE ONLY)

FUTURE WEARING SURFACE (FWS) OF 0.60 KIPS/SQ.FT.

## DESIGN DATA

CONCRETE CLASS, QSC2 - COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE)

CONCRETE CLASS, QSC1 - COMPRESSIVE STRENGTH 4.0 KSI (SUBSTRUCTURE)

REINFORCING STEEL - MINIMUM YIELD STRENGTH 60 KSI

DECK PROTECTION METHOD

EPOXY COATED REINFORCING STEEL

2.5" CONCRETE COVER

MONOLITHIC WEARING SURFACE

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1" THICK.

# ASBESTOS NOTIFICATION

AN ASBESTOS SURVEY OF THE BRIDGE WAS CONDUCTED BY A CERTIFIED ASBESTOS EVALUATION SPECIALIST. THE SURVEY DETERMINED THAT NO ASBESTOS IS PRESENT AT THE BRIDGES.

A COPY OF THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) NOTIFICATION OF DEMOLITION AND RENOVATION FORMS, PARTIALLY COMPLETED AND SIGNED BY THE BRIDGE OWNER, WILL BE PROVIDED TO THE SUCCESSFUL BIDDER. THE CONTRACTOR SHALL COMPLETE THE FORM AND SUBMIT IT TO:

OHIO EPA, COD 50 WEST TOWN ST, SUITE 700 COLUMBUS, OHIO 43215 KELLY TOTH, APC MANAGER (614) 728-3898

AT LEAST TEN (10) DAYS PRIOR TO THE START OF ANY DEMOLITION AND/OR REHABILITATION. THE CONTRACTOR SHALL PROVIDE A COPY OF THE COMPLETED FORM TO THE

INFORMATION REQUIRED ON THE FORM WILL INCLUDE: INFORMATION REQUIRED ON THE FORM WILL INCLUDE:

1) THE CONTRACTOR'S NAME AND ADDRESS, 2) THE

SCHEDULED DATES FOR THE START AND COMPLETION OF

THE BRIDGE REMOVAL, AND 3) A DESCRIPTION OF THE

PLANNED DEMOLITION WORK AND THE METHOD(S) TO BE

THE CONTRACTOR SHALL FURNISH ALL FEES, LABOR, AND MATERIAL NECESSARY TO COMPLETE AND SÚBMIT THE OEPA NOTIFICATION FORM.

# DECK PLACEMENT DESIGN ASSUMPTIONS

THE FOLLOWING ASSUMPTIONS OF MEANS AND METHODS WERE MADE FOR THE ANALYSIS AND DESIGN OF THE SUPERSTRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE FALSEWORK SUPPORT SYSTEM WITHIN THESE PARAMETERS AND WILL ASSUME RESPONSIBILITY FOR THE SUPERSTRUCTURE ANALYSIS FOR DEVIATION FROM THESE DESIGN ASSUMPTIONS.

AN EIGHT WHEEL FINISHING MACHINE WITH A MAXIMUM WHEEL LOAD OF 2.55 KIPS

A MINIMUM OUT-TO-OUT WHEEL SPACING AT EACH END OF THE MACHINE OF 103"

A MAXIMUM SPACING OF OVERHANG FALSEWORK BRACKETS OF

A MAXIMUM DISTANCE FROM THE CENTERLINE OF THE FASCIA GIRDER TO THE FACE OF THE SAFETY HANDRAIL OF 65"

# EXISTING STRUCTURE VERIFICATION

DETAILS AND DIMENSIONS SHOWN ON THESE PLANS PERTAINING TO THE EXISTING STRUCTURE HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND FROM FIELD OBSERVATIONS AND MEASUREMENTS CONSEQUENTLY, THEY ARE INDICATIVE OF THE EXISTING STRUCTURE AND THE PROPOSED WORK, BUT THEY SHALL BE CONSIDERED TENTATIVE AND APPROXIMATE. THE CONTRACTOR IS REFERRED TO CMS SECTIONS 102.05. 105.02, AND 513.04. BASE CONTRACT BID PRICES UPON A RECOGNITION OF THE UNCERTAINTIES DESCRIBED ABOVE AND UPON A PREBID EXAMINATION OF THE EXISTING STRUCTURE. HOWEVER, THE DEPARTMENT WILL PAY FOR ALL PROJECT WORK BASED UPON ACTUAL DIMENSIONS WHICH HAVE BEEN VERIFIED IN THE FIELD.

ITEM 202 - PORTIONS OF STRUCTURE REMOVED, AS PER PLAN

DESCRIPTION: THIS WORK CONSISTS OF THE REMOVAL OF THE SUBSTRUCTURE AND CONCRETE DECK INCLUDING PARAPETS FROM THE STEEL SUPPORTING SYSTEMS
(GIRDERS, CROSSFRAMES, ETC.), SCUPPERS, EXPANSION
JOINT, EXPANSION JOINT ARMOR, END CROSS FRAMES, AND
INCIDENTALS. THE PROVISIONS OF ITEM 202 APPLY
EXCEPT AS SPECIFIED BY THE FOLLOWING NOTES. PERFORM
WORK CAREFULLY DURING DECK REMOVALS TO PROTECT PORTIONS OF SUCH SYSTEMS THAT ARE TO BE SALVAGED AND INCORPORATED INTO THE PROPOSED STRUCTURE. THE USE OF EXPLOSIVES, HEADACHE BALLS, AND/OR HOE RAM TYPE EQUIPMENT IS PROHIBITED. SUBMIT CONSTRUCTION PLANS ACCORDING TO CMS 501.05.

PROTECTION OF TRAFFIC: PRIOR TO DEMOLITION OF ANY PORTIONS OF THE EXISTING SUPERSTRUCTURE, THE CONTRACTOR SHALL SUBMIT PLANS FOR THE PROTECTION OF TRAFFIC (VEHICULAR, PEDESTRIAN, BOAT, ETC.) AS PER CMS 2019 01.05.B.2.

PROTECTION OF STEEL SUPPORT SYSTEMS: BEFORE DECK SLAB CUTTING IS PERMITTED, DRAW THE OUTLINE OF PRIMARY STEEL MEMBERS IN CONTACT WITH THE BOTTOM STEEL MEMBERS THAT ARE TO BE INCORPORATED INTO THE PROPOSED STRUCTURE. REPLACE OR REPAIR STEEL MEMBERS DAMAGED BY THE DECK SLAB CUTTING
OPERATIONS AT NO COST TO THE PROJECT. AT LEAST 7
DAYS BEFORE PERFORMING REPAIR WORK, SUBMIT A
PROPOSED REPAIR PLAN DEVELOPED BY AN OHIO
REGISTERED PROFESSIONAL ENGINEER TO THE DIRECTOR.
OBTAIN THE DIRECTOR'S APPROVAL BEFORE PERFORMING

REMOVAL METHODS: THE CONTRACTOR MAY REMOVE
CONCRETE BY CUTTING AND BY MEANS OF HAND OPERATED
PNEUMATIC HAMMERS EMPLOYING POINTED OR BLUNDED
CHISEL TYPE TOOLS. FOR REMOVALS OVER STRUCTURAL
MEMBERS (STEEL GIRDERS), THE CONTRACTOR MAY USE A
HAMMER HEAVIER THAN 35 POUNDS BUT NOT TO EXCEED 90 POUNDS UNLESS APPROVED BY THE ENGINEER. REMOVAL METHODS OVER STRUCTURAL MEMBERS SHALL ENSURE ADEQUATE DEPTH CONTROL AND PREVENT NICKING OR GOUGING TH EPRIMARY STRUCTURAL MEMBERS.

DUE TO THE POSSIBLE PRESENCE OF ATTACHMENTS (E.G., FINISHING MACHINE, SCUPPER AND FORM SUPPORTS, ETC.) TO EXISTING STRUCTURAL MEMBERS, PERFORM WORK CAREFULLY DURING DECK REMOVAL TO AVOID DAMAGING STRUCTURAL MEMBERS THAT ARE TO REMAIN. REPLACE OR STRUCTURAL MEMBERS THAT ARE TO REMAIN. REPLACE OF REPAIR STRUCTURAL MEMBERS DAMAGED BY THE REMOVAL OPERATIONS AT NO COST TO THE PROJECT. AT LEAST TO AYS BEFORE PERFORMING THE REPAIR WORK, SUBMIT A PROPOSED REPAIR PLAN, DEBELOPED BY AN OHIO REGISTERED PROFESSIONAL ENGINEER TO THE DIRECTOR. OBTAIN THE DIRECTOR'S APPROVAL BEFORE PERFORMING THE DEPAIR THE REPAIR.

### INSPECTION OF EXISTING STEEL

THE ENGINEER WILL VISUALLY INSPECT ALL EXISTING BUTT-WELDED SPLICES AND/OR TOP FLANGE COVER PLATE FILLET WELDS TO ENSURE THE WELDS, PLATES, AND GIRDERS ARE FREE OF DEFECTS AND CRACKS. IF NECESSARY, REMOVE ALL DECK SLAB HAUNCH FORMS IMMEDIATELY ADJACENT TO SUCH WELDS THAT MAY IMMEDIATELY ADDACENT TO SOCH WELDS THAT MAT INTERFERE WITH THE ENGINEER'S INSPECTION. THE INSPECTION WILL NOT TAKE PLACE UNTIL THE TOP FLANGES ARE CLEANED ACCORDING TO 511.10, BUT IT WILL BE DONE BEFORE THE DECK SLAB REINFORCEMENT IS INSTALLED. THE ENGINEER WILL REPORT ALL CRACKS FOUND TO THE ENGINEER WILL REPORT ALL CRACKS FOUND TO THE OFFICE OF CONSTRUCTION ADMINISTRATION, BRIDGE CONSTRUCTION SPECIALIST, ALONG WITH SPECIFIC INFORMATION ON LOCATION OF THE CRACKS, LENGTH, AND DEPTH SO AN EVALUATION AND REPAIR OR REPLACEMENT RECOMMENDATION CAN BE MADE.

# EXISTING WELDED ATTACHMENTS

REMOVE EXISTING WELDED ATTACHMENTS (E.G., FINISHING MACHINE AND FORM SUPPORTS, AND SUPPORTS FOR SCUPPERS AND BULB ANGLES WHICH ARE TO BE REMOVED) LOCATED IN THE DESIGNATED TENSION PORTIONS OF THE TOP FLANGES OF EXISTING STEEL MEMBERS AND GRIND THE FLANGE SURFACES SMOOTH. CAREFULLY GRIND PARALLEL TO THE FLANGES.

### MEASUREMENT AND PAYMENT

THE DEPARTMENT WILL PAY FOR THE ACCEPTED QUANTITIES OF REMOVALS AT THE CONTRACT BID PRICE FOR ITEM 202, PORTIONS OF STRUCTURE REMOVED, AS PER PLAN.

# SUBSTRUCTURE CONCRETE REMOVAL

ALL CONCRETE REMOVED IN THE PLANS SHALL BE REMOVED BY MEANS OF CUTTING AND OF APPROVED PNEUMATIC HAMMERS EMPLOYING POINTED AND BLUNT CHISEL TOOLS HYDRUALIC HOE-RAM TYPE HAMMERS WILL NOT BE PERMITTED. THE WEIGHT OF THE HAMMER SHALL NOT BE MORE THAN 35 POUNDS FOR REMOVAL WITHIN 18 INCHES OF PORTIONS TO BE PRESERVED. OUTSIDE THE 18 INCH LIMIT, THE CONTRACTOR MAY USE HAMMERS NOT EXCEEDING 90 POUNDS UPON THE APPROVAL OF THE ENGINEER. DO NOT PLACE PNEUMATIC HAMMERS IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE REBUILT STRUCTURE.

ITEM 516 - 2" DEEP JOINT SEALER, AS PER PLAN

UPON COMPLETION OF THE PROPOSED APPROACH SLAB THE CONTRACTOR SHALL SAW CUT ALONG THE APPROACH SLAB AND BRIDGE LIMIT, AS DETAILED IN THE PLANS, AN AREA 1" WIE BY 2" DEEP AND FILL THIS AREA WITH HOT APPLIED JOINT SEALER 705.04.

ITEM 516 - JACKING AND TEMPORARY SUPPORT OF SUPERSTRUCTURE, AS PER PLAN

THIS WORK CONSISTS OF RAISING OR RE-POSITIONING EXISTING STRUCTURES TO THE DIMENSIONS AND REQUIREMENTS DEFINED IN THE PROJECT PLANS. SUBMIT CONSTRUCTION PLANS IN ACCORDANCE WITH 501.05.

IF, DURING THE JACKING OPERATIONS, CRACKING OF THE IF, DURING THE JACKING OPERATIONS, CHACKING OF THE CONCRETE SUPERSTRUCTURE, SEPARATION OF THE CONCRETE DECK FROM THE STEEL STRINGERS, OR OTHER DAMAGE TO THE STRUCTURE IS VISUALLY OBSERVED, IMMEDIATELY CEASE THE JACKING OPERATION AND INSTALL SUPPORTS TO THE SATISFACTION OF THE ENGINEER. ANALYZE THE DAMAGE AND SUBMIT A METHOD OF CORRECTION TO THE ENGINEER FOR APPROVAL.

EPOXY INJECT ALL BEAMS THAT SEPARATE FROM THE DECK FOR THE DISTANCE OF SEPARATION IN ACORDANCE WITH CMS 512.07. THE DEPARTMENT WILL NOT PAY FOR THE COST OF THIS INJECTION OR OTHER REQUIRED REPAIRS. THE BRIDGE BEARINGS SHALL BE FULLY SEATED AT ALL CONTACT AREAS. IF FULL SEATING IS NOT ATTAINED, SUBMIT A REPAIR PLAN TO THE ENGINEER . THE DEPARTMENT WILL NOT PAY FOR THE REPAIR COSTS TO ENSURE FULL SEATING ON THE BEARINGS.

THE DEPARTMENT WILL MEASURE THIS WORK ON A LUMP

THE DEPARTMENT WILL PAY FOR THE ACCEPTED QUANTITIES AT THE CONTRACT PRICE FOR ITEM 516 - JACKING AND TEMPORARY SUPPORT OF SUPERSTRUCTURE, AS PER PLAN.

# DECK SLAB CONCRETE QUANTITY

THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS BASED ON THE CONSTANT DECK SLAB THICKNESS, AS SHOWN, PLUS THE QUANTITY OF CONCRETE THAT FORMS EACH BEAM HAUNCH. THE ESTIMATE ASSUMES A CONSTANT HAUNCH THICKNESS OF 3". DEVIATE FROM THIS HAUNCH THICKNESS AS NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. THE ALLOWABLE TOLERANCE FOR THE WARNEY WITH A CONTROLLED THE PLANTER OF THE PLANTER THE FINISHED GRADE. THE ALLOWABLE TOLERANCE FOR THE HAUNCH WIDTH OUTSIDE THE EDGE OF EACH BEAM FLANGE IS 3 INCHES. THE HAUNCH THICKNESS WAS MEASURED AT THE CENTERLINE OF THE BEAM, FROM THE SURFACE OF THE DECK TO THE BOTTOM OF THE TOP FLANGE MINUS THE DECK SLAB THICKNESS, THE AREA OF ALL EMBEDDED STEEL PLATES HAS BEEN DEDUCTED FROM THE HUANCH QUANTITY IN ACCORDANCE WITH 511.24.

ITEM 513 - STRUCTURAL STEEL, MISC .: 2" DIA. FIELD DRILLED HOLES

THIS ITEM SHALL CONSIST OF FIELD LOCATING AND FIELD DRILLING OF 2" DIAMETER HOLES FOR #8 REINFORCING STEEL AS PER STD. DWG. SICD-1-96 INTO THE EXISTING BEAM ENDS. THE CONTRACTOR SHALL CLEARLY MARK THE LOCATIONS AND HAVE THE ENGINEER'S APPROVAL PRIOR TO DRILLING. FLAME CUTTING OF THE HOLES WILL NOT BE

MEASUREMENT: THE DEPARTMENT WILL PAY FOR EACH 2" DIA. HOLE DRILLED.

ALL MATERIALS, LABOR, AND INCIDENTALS TO PERFORM THE ABOVE WORK SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 513 - STRUCTURAL STEEL, MISC.: 2" DIA. FIELD

ITEM 514 - FIELD PAINTING OF EXISTING STRUCTURAL STEEL, PRIME COAT, AS PER PLAN

ALL PROVISIONS OF 514 SHALL APPLY FOR THE APPLICATION OF PRIME COAT WITHT HE FOLLOWING PROVISIONS. THE CONTRACTOR SHALL USE INTERNATIONAL PAINT INTERZINC 52, ORGANIC ZINC, PRIMER AS REFERENCED IN ODOT'S QUALIFIED PRODUCTS LIST (QPL) FOR 708.02.

ITEM 514 - FIELD PAINTING OF EXISTING STRUCTURAL STEEL, INTERMEDIATE COAT, AS PER

ALL PROVISIONS OF 514 SHALL APPLY FOR THE APPLICATION OF INTERMEDIATE COAT WITH THE FOLLOWING PROVISIONS. THE CONTRACTOR SHALL USE INTERNATIONAL PAINT INTERGUARD 475 HS, EPOXY, INTERMEDIATE COAT AS REFERENCED IN ODOT'S QUALIFIED PRODUCTS LIST (QPL) FOR 708.02.

ITEM 514 - FIELD PAINTING OF EXISTING STRUCTURAL STEEL, FINISH COAT, AS PER PLAN, URETHANE

ALL PROVISIONS OF 514 SHALL APPLY FOR THE APPLICATION OF FINISH COAT WITH THE FOLLOWING PROVISIONS. THE CONTRACTOR SHALL USE INTERNATIONAL PAINT INTERTHANE 990 HS, URETHANE, FINISH COAT AS REFERENCED IN ODOT'S QUALIFIED PRODUCTS LIST (QPL) FOR 708.02.

OF 'RICT OHIO DEPARTMENT TRANSPORTATION DIST

**NOTES** R-23-0185 BRIDGE NC
10. MAR-23
70. SR 5

> 9 ญ่ 102332 23-1.85 Š

MAR 3 /30

PID



ITEM 514 - FIELD PAINTING OF EXISTING STRUCTURAL STEEL, FINISH COAT, AS PER PLAN, POLYSILOXANE

ALL PROVISIONS OF 514 SHALL APPLY FOR THE APPLICATION OF FINISH COAT WITH THE FOLLOWING PROVISIONS. THE CONTRACTOR SHALL USE INTERNATIONAL PAINT INTERFINE 979, POLYSILOXANE AS DEFINED IN 514.17.E. PRIOR TO APPLICATION, THE CONTRACTOR SHALL CHECK FOR CURE, DRY FILM THICKNESS MEASUREMENTS, AND CLEANLINESS OF THE PRIME COAT. STRIPE COAT INTERNATIONAL PAINT INTERFINE 979, POLYSILOXANE AS DEFINED IN 514.17.E. APPLY THE COATING TO A DRY FILM THICKNESS MEASUREMENT OF 4 TO 6 MILS. AFTER CURE, CAULK GAPS OR CREVICES UP TO 1/2 INCH (13MM) WIDE. FOLLOW THE MANUFACTURER;S RECOMMENDATIONS FOR CURING BEFORE APPLYING ADDITIONAL MILS OF POLYSILOXANE TO COVER EXPOSED CAULK.

PAINTING OF STRUCTURAL STEEL

ALL STRUCTURAL STEEL SHALL BE PAINTED IN ACCORDANCE WITH SECTION 514 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS. THE FINISH COAT COLOR SHALL BE FS-595C, 14277 (GREEN).

ITEM 519 - PATCHING CONCRETE STRUCTURE

THE FOLLOWING ITEMS HAVE BEEN PROVIDED AS CONTINGENCY OUANTITIES TO BE USED AS DIRECTED BY THE ENGINEER AT THE NOTED LOCATIONS. THESE ITEMS HAVE BEEN CARRIED TO THE STRUCTURES SUMMARY.

MAR-23-0185L

 $\bigcirc$ 

 $\bigcirc$ 

ITEM 519 - PATCHING CONCRETE STURCTURE = 50 SF

MAR-23-0185R

ITEM 519 - PATCHING CONCRETE STRUCTURE = 50 SF

ABUTMENT DIAPHRAGM CONCRETE

PLACE THE DIAPHRAGM CONCRETE ENCASING THE STRUCTURAL MEMBER ENDS AFTER THE DECK PLACEMENT IN THE ADJACENT SPAN IS COMPLETE. PROCEDURES THAT PLACE THE ABUTMENT DIAPHRAGM WITH THE DECK CONCRETE MAY BE APPROVED BY THE ENGINEER IF THE PLACEMENT SUBMITTAL CAN ASSURE THAT THE DECK CONCRETE IN THE ADJACENT SPAN WILL BE PLACED BEFORE CONCRETE IN THE DIAPHRAGM HAS REACHED ITS INITIAL SET.

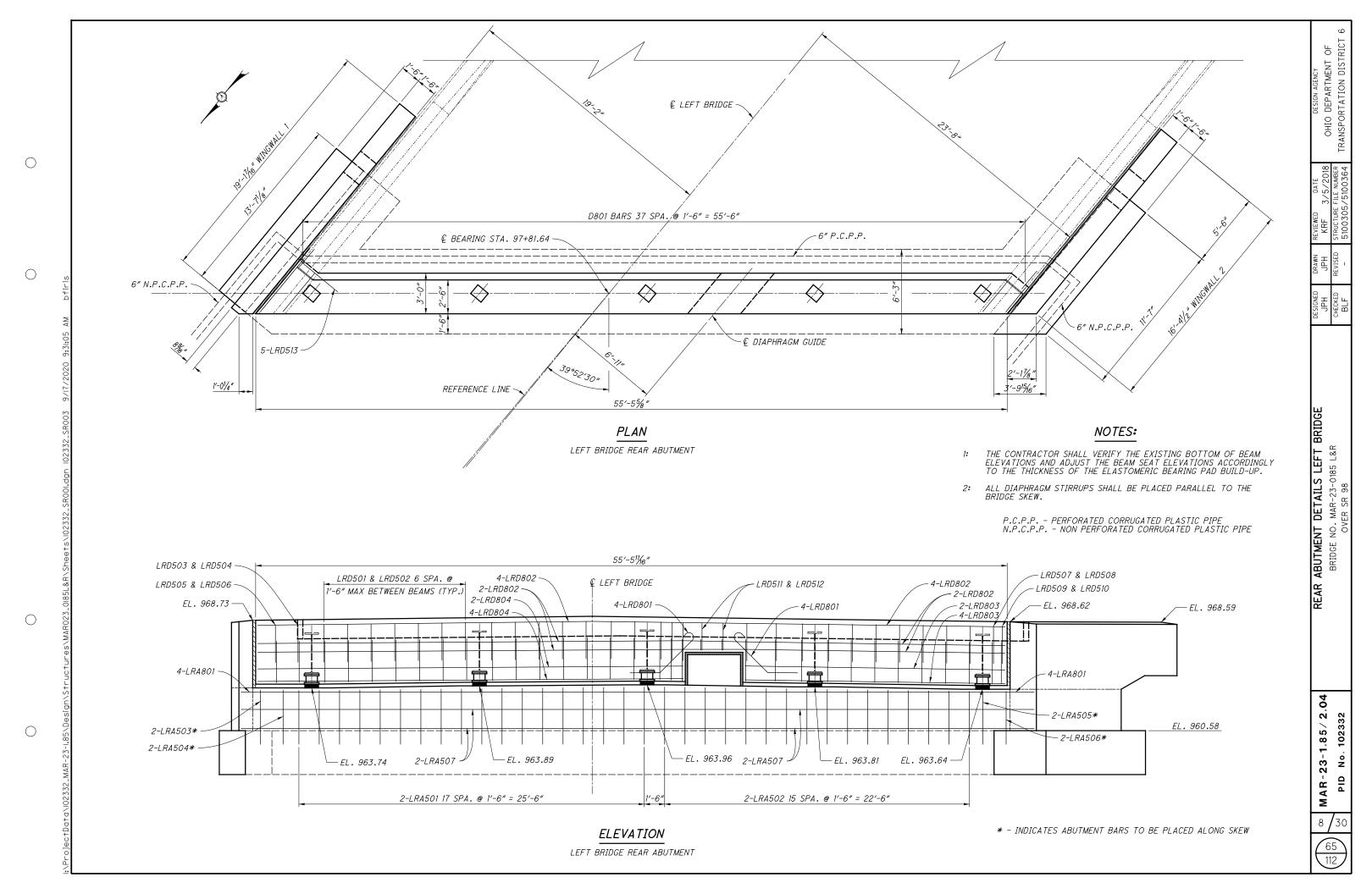
OF TRICT DESIGN AGENCY
OHIO DEPARTMENT C
TRANSPORTATION DISTR 9 ญ่ MAR-23-1.85/ Š PID

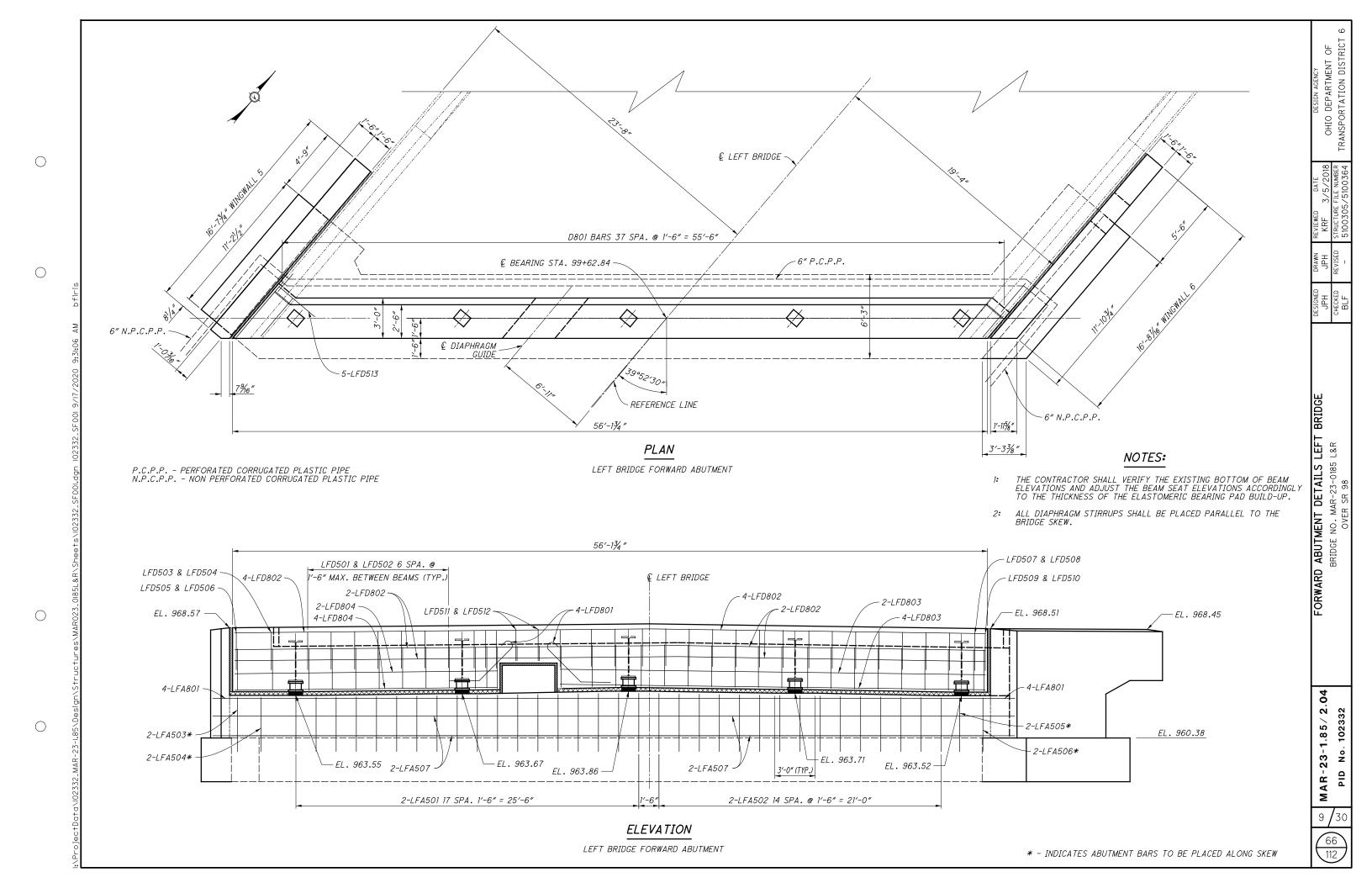
18						ESTIMATED QUANTITIES (MAR-23-0185L SFN: 5100305)								ESTIMATED QUANTITIES (MAR-23-0185R SFN: 5100364)				
Company   Comp	ЕМ	EXTENSION			UNIT	DESCRIPTION ABUT. PIERS	SUPER.	. GEN.	SHEET #	# ITEM	EXTENSION		UNIT	DESCRIPTION ABUT.	PIERS	SUPER.	GEN. SHEE	:T #
10   10   10   10   10   10   10   10			17.4					17.4				17.4						′30
1000   13   15   MACESTEE SECURITION   1,000																		$\dashv$
1000   1000		11100			15	COEFFERING AND EVENUATION PROCING				507	11100		1.5	COEFFERANC AND EVEN ATTOM PRACTICE				=
2000   227   EAN   SOUTH SECURITY STREETING FOR STATE AND STATE STREETING																		
2000   277   445   506   505   506   279   506		10000	91.6	30	I R	FPOYY COATED REINFORCING STEEL 5.851	85 780	)		509	10000	90 421	I R	FPOXY COATED REINFORCING STEEL 5.679		84 742		_
250   250							00,700									01,112		
2580   2   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   7   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   7   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   7   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   7   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   7   C.C.   C.C.   SER STEER BLUFFERS MORE, SER FR. M.   SER STEER BLUFFERS MORE, SER FR. M.   SER STEER BLUFFERS MORE, SER		10000	221		EACH	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT				510	10000	208	EACH	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT 208				=
3488   72   CT   DASC COCCURSTS, SIGNES OF CHARMEN   17   58   34448   72   CT   CLASS DICKNORM, SIGN PHARMEN   17			_						16 (70								16.4	/70
1000   169   55   SALING OF CONCRETE SIMPLESS						CLASS QC2 CONCRETE, BRIDGE DECK (PARAPET)	_		16730								167	/30
2,500   2,50		45710	83	-	CY	CLASS QCI CONCRETE, ABUTMENT 83				511	45710	83	CY	CLASS QCI CONCRETE, ABUTMENT 83				_
\$60.00   \$0.		10050	619		SY	SEALING OF CONCRETE SURFACES (NON-EPOXY)		619		512	10050	619	SY	SEALING OF CONCRETE SURFACES (NON-EPOXY)			619	
\$60.00   \$0.		20000	2.98	10	EACH	WELDED STUD SHEAR CONNECTORS	2.980			513	20000	2,980	EACH	WELDED STUD SHEAR CONNECTORS		2.980		_
DOCST   9,003   SF   FELD PARTING OF EXISTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   9,809   4/30   5H   00005T   9,808   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   9,809   4/30   5H   00005T   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   9,809   4/30   5H   00005T   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,809   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME   9,800   SF   FELD PARTING STRUCTURAL STEEL, PRING COST, AS FOR PLAN   POLYCOME					EACH	STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED HOLES			3/30	513	95030		EACH	STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED HOLES			3/.	′30
0006  9,869   SF   FILD PANTING STRUCTURAL STELL, NISMEGIATE COAT, AS PER PLAN   9,869   470   5M   0006  9,860   SF   FILD PANTING STRUCTURAL STELL, PIGNET COAT, AS PER PLAN, POLYSLOWINE   9,868   470   5M   0006  33   MARW GROUNDS FINGE FIRES, SUPERS ON USES TRUCTURAL STELL   33   5M   0006  33   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   33   5M   0006  35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   33   5M   0006  35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   33   5M   0006  35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   33   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   33   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW GROUNDS FIRES, FLANS, SUPERS ON USES TRUCTURAL STELL   35   MARW G		00050	10,00	03	SF	SURFACE PREPARATION OF EXISTING STRUCTURAL STEEL	10,003	3		514	00050	10,101	SF	SURFACE PREPARATION OF EXISTING STRUCTURAL STEEL		10,101		$\dashv$
DODGET   9,869   SF   FEED PAIRTING STRUCTURAL STEEL, FROMS FOOD, AS FER PLAN, INCREMENT   9,889   4/30   514   00000   15   50000																		/30 /30
0000   33   MARR   ORDING FIRS, TEARS, SURES ON EXISTING STRUCTURAL STEEL   33   5M   NOOD   15   EACH   FURL INSPECTION REPAIR   10   10   10   10   10   10   10   1	_					FIELD PAINTING STRUCTURAL STEEL, FINISH COAT, AS PER PLAN, URETHANE					00504		MNHR	GRINDING FINS, TEARS, SLIVERS ON EXISTING STRUCTURAL STEEL			4/.	′30
100	7							1		514	10000	15	EACH			15		-
15900   2   5   FREFORKE DEPASION JOINT FILLER     2   5   18   1900   17   5   2 FREFORKE DEPASION JOINT FILLER     17   18   18   18   19   19   19   19   19							10											彐
1930   17   5F   2* PREFORMED EXPANSION JOINT FILLER   17   516   M020   H4   FT   SEMI-INTEGRAL EXPANSION JOINT SEAL   10   10   10   10   10   10   10   1																		_
3301   103   FT   2*DEP JOINT SEALER, AS PER PLAN   103   3.30   5.68   4100   20   EACH   109* PREFORMED BEARING PAD   20   4400   10   EACH   109* PREFORMED BEARING PAD   100			17		SF	2" PREFORMED EXPANSION JOINT FILLER		17		516	14020	144	FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL			144	
### AUDITION OF THE PREFORMED BEARING PAD ### AUDITION OF THE PROFORMED BEARING PAD ### AUDITION OF THE PREFORMED BEARING PAD ### AUDITOR PAD *## AUDITOR PAD *## AUDITOR PAD *** AUDITOR PAD *** AUDI							10.3		3/30	516	31011	103	FT	2" DEEP JOINT SEALER, AS PER PLAN		103	3/.	′30
44200   10   EACH   ELASTOMERIC BEARING WITH INTERNAL LAMIMATES AND LOAD PLATE INCOPRENE] 102*NO*3.247?   10   516   44200							1,00											
47001   LS   JACKING AND TEMPORARY SUPPORT OF SUPERSTRUCTURE, AS PER PLAN   3/30   5/8   2/200   59   CY   POROUS BACKFILL WITH GEOTEXTILE FABRIC   5/9   5/18   40000   1/3   FT   6" PERFORATED CORRUGATED PLASTIC PIPE   1/12   1/2   5/18   40010   62   FT   6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS   6/2																		-
21200   59   CY   POROUS BACKFILL WITH GEOTEXTILE FABRIC   59   518   40000   13   FT   6" PERFORATED CORRUGATED PLASTIC PIPE   112   518   40000   13   FT   6" PERFORATED CORRUGATED PLASTIC PIPE   12   518   40010   62   FT   6" NON-PERFORATED CORRUGATED PLASTIC PIPE   1100   50   5F   PATCHING CONCRETE STRUCTURE   9   1/30   1/30   50   5F   PATCHING CONCRETE STRUCTURE   9   1/30   526   30000   286   SY   REINFORCED CONCRETE APPROACH SLABS (T=17)   110			10					10	7.770	516	47001		LS	JACKING AND TEMPORARY SUPPORT OF SUPERSTRUCTURE, AS PER PLAN			3/.	′30
40000   112   FT   6" PERFORATED CORRUGATED PLASTIC PIPE   112   518   40010   62   FT   6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS   62		4/001	$\pm$		LS	JACKING AND TEMPORART SUPPORT OF SUPERSTRUCTURE, AS PER PLAN		$\pm$	3/30	518	21200	59	CY	POROUS BACKFILL WITH GEOTEXTILE FABRIC			59	
40010   62   FT   6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS   62   519   11100   50   5F   PATCHING CONCRETE STRUCTURE   50   50   50   55   55   50   55   55   50   55   55   50   55																		=
11100   9   SF   PATCHING CONCRETE STRUCTURE   9   1/30																		
11100   50   SF   PATCHING CONCRETE STRUCTURE   50   4/30   526   30000   286   SY   REINFORCED CONCRETE APPROACH SLABS (T=17")   286	$\dashv$	11100	.0	$-\Gamma$	SF	PATCHING CONCRETE STRUCTURE			1/30	519	11100	50	SF	PATCHING CONCRETE STRUCTURE			50 4/.	30
30000 286 SY REINFORCED CONCRETE APPROACH SLABS (T=17°) 286 90030 112 FT TYPE C INSTALLATION 112 607 39900 340 FT VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC 39900 340 FT VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC 4	1							50										
90030         112         FT         TYPE C INSTALLATION         112         607         39900         340         FT         VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC         340           39900         340         FT         VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC         4         EACH         PRECAST REINFORCED CONCRETE OUTLET         4		30000	286	<del>,</del>	SY	REINFORCED CONCRETE APPROACH SLABS (T=17")		286		526	90030	111	FT	TIPE C INSTALLATION			111	
		90030	112		FT	TYPE C INSTALLATION		112		607	39900	340	FT	VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC		340		=
997/0 4 5409 PRELAST REPRODUCE CONDUCT UTLET 4		39900	340	)	FT	VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC	340			611	99710	4	EACH	PRECAST REINFORCED CONCRETE OUTLET			4	
		99710	4		EACH	PRECAST REINFORCED CONCRETE OUTLET		4										$\dashv$

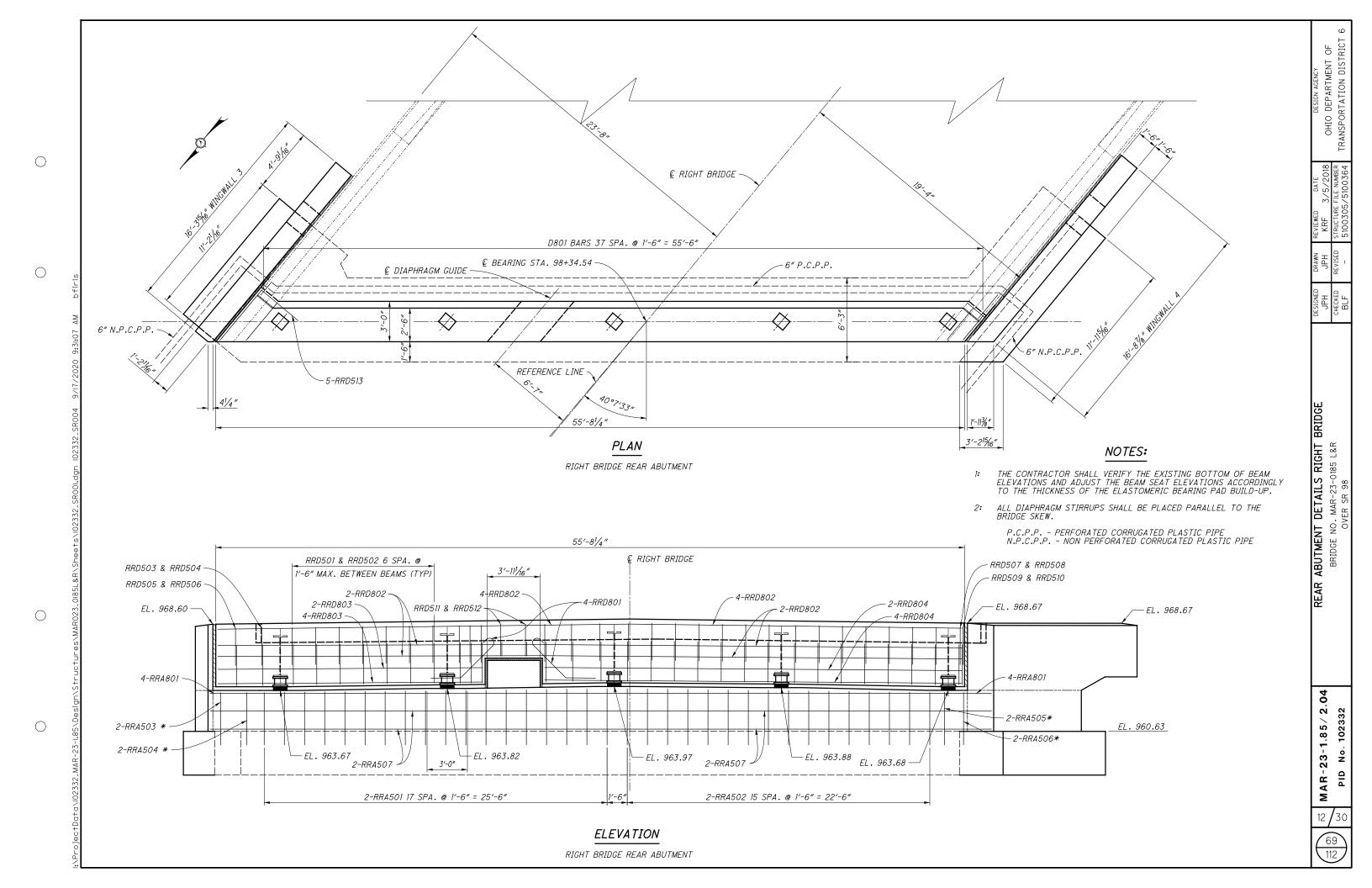
 $\bigcirc$ 

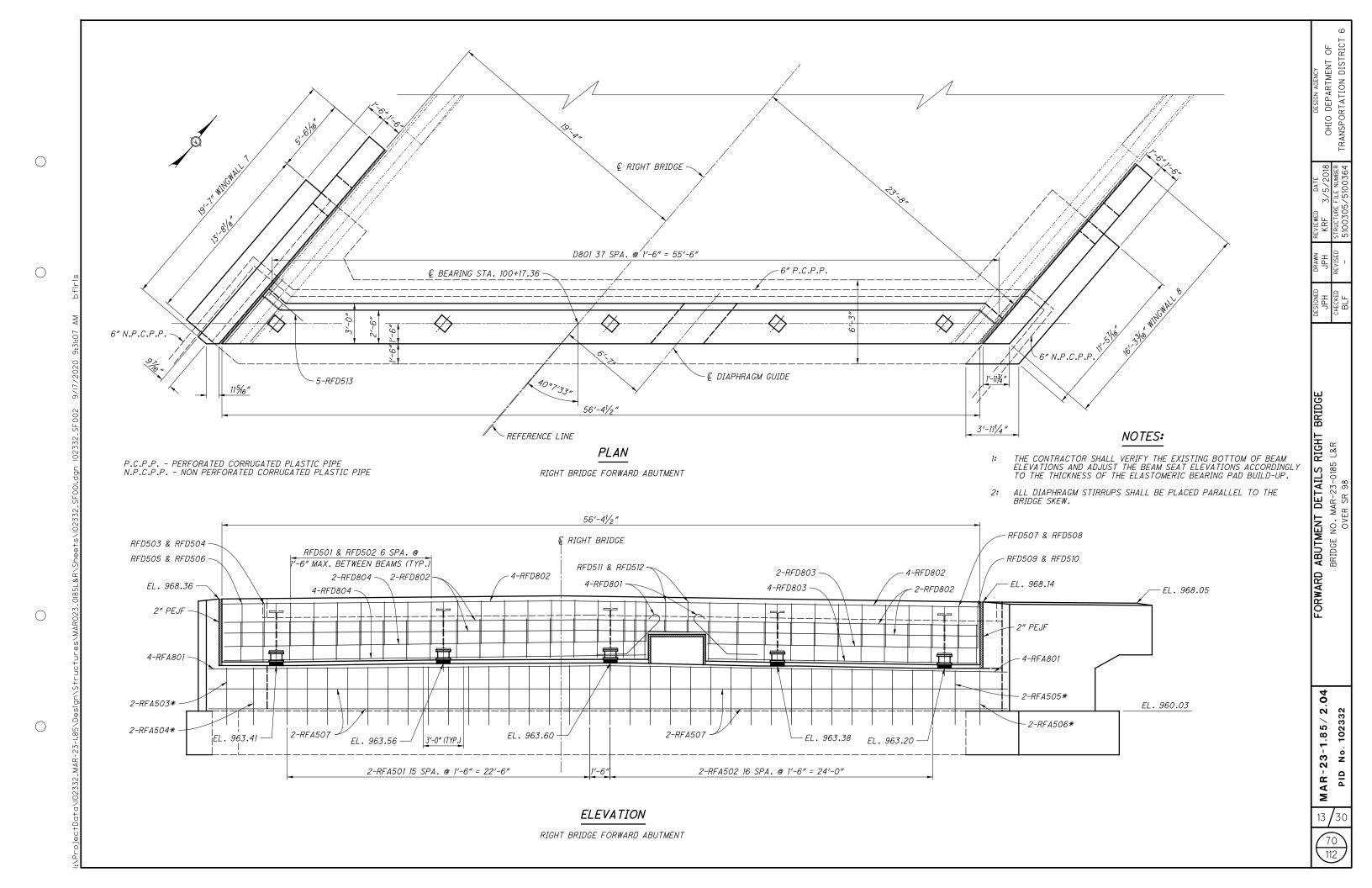
 $\bigcirc$ 

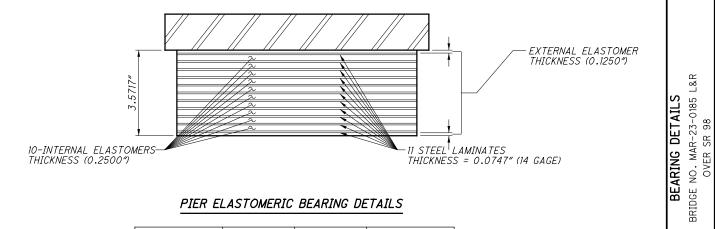
 $\bigcirc$ 











# PIER ELASTOMERIC BEARING DETAILS

	D.L. (KIPS)	L.L. (KIPS)	TOTAL (KIPS)
PIER 1&2	<i>115</i>	64	179

	DIM. "H"
PIER 1	NO UPPER LOAD PLATE
PIER 2	<i>3.9283″</i>

# NOTES:

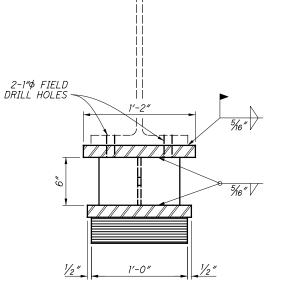
# ELASTOMERIC BEARINGS

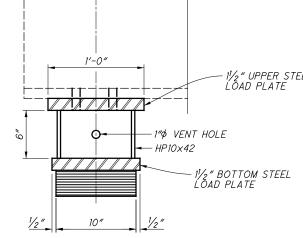
THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION 1, SECTION 14.6.6 (METHOD A) OF THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES

# LOAD PLATE

THE STEEL LOAD PLATES AND HP 10×42 SHALL BE MADE OF A709 STEEL. SURFACE PREPARATION AND PRIMING SHALL BE DONE IN THE SHOP AND REQUIRED FIELD DRILLED HOLES SHALL BE INCLUDED IN THE PRICE BID ITEM 516 ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATES (NEOPRENE)

THE STEEL LOAD PLATE SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER DURING THE MOLD PROCESS.

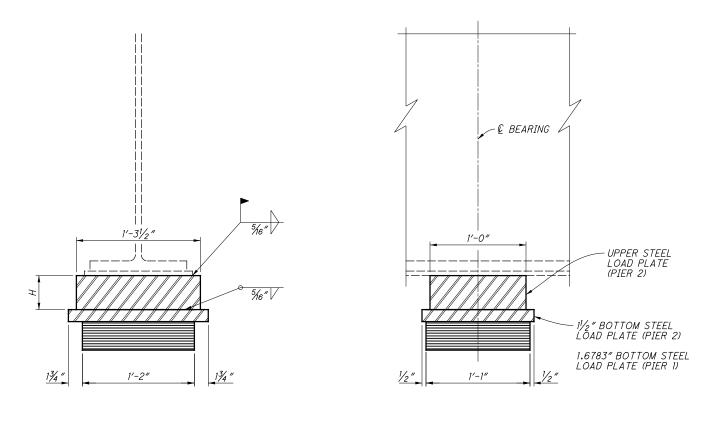




PERPENDICULAR TO @ OF BEARING

PARALLEL TO & OF BEARING

# ABUTMENT BEARING DETAILS



PERPENDICULAR TO & OF BEARING

PARALLEL TO & OF BEARING

PIER BEARING DETAILS

 $\bigcirc$ 

MAR-23-1.85/2.04

OF TRICT

OHIO DEPARTMENT TRANSPORTATION DIST

*TYPE-37* 

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

REINFORCING STEEL LIST BRIDGE NO. MAR-23-0185 L&R OVER SR 98

DESIGN AGENCY
OHIO DEPARTMENT OF
TRANSPORTATION DISTRICT 6

MAR-23-1.85/2.04 PID No. 102332

**2**7/30

84

TYPE-38

				l F	FT BR	IDGF	DECK	,						—
S401	240	40'-0"	-0"	6,413	STR		DEUN		1	$\overline{}$	$\neg$	$\neg \vdash$	$\overline{}$	-
S402	60	33′-7′′	-7′′	1,346	STR									
S403	192	2′-0″		257	1			1'-6''	1				$-\!\!\perp$	
S404	192	2'-3"		289	19			0'-8"	0'-4''	+	_	+	-+	
S405 S406	192 192	1'-10'' 2'-0''		235 257	19		'-8'' '-3''	1'-3'' 0'-8''	0'-4"	+	-	-	-+	
3400	132	2 0	<u> </u>	201	13	+ '		0 0	1 7			-	-+	
	1	2'-4''					′-6′′							
S501	SERIES OF			1,491	16		TO						[ (	0′-6.9′′
	67	40'-4"			+	39	9'-6''		1	+	$\perp$	+	-+	$\longrightarrow$
S502	SERIES OF	1'-6'' TO		1,450	STR	1								0′-6.9″
3302	67	39'-6"		טטדקו	318	1							Ι,	0.3
S503	319	20'-10"		6,932	16	20	0'-0"		1				-	
S504	319	20'-0''		6,654	STR									
S505	2	24'-4''	-4''	51	16	23	3′-6′′							
S506	2	23′-6″		49	STR				1			-	$\perp$	
S507	2	24'-10"		52	16 STD		1'-0''		-	+	+	-	$\rightarrow$	
S508 S509	2 315	24'-0'' 26'-9''		50 8,789	STR 16		5′-11′′		1	-	-	-	-+	
S510	315	25'-9''		8,789	STR		, "		+	+	+	-	+	
S511	472	40'-0"		19,692	STR				1	1	1	-	-+	
	1	2'-3''		,	1		′-5″		1	1		$\neg$	-+	
S512	SERIES OF	TO	)	1,513	16	7	то						1 (	0′-6.8′′
	68	40′-5′′				39	9′-7′′		1			$\perp$	$\longrightarrow$	
CE17	1 SERTES OF	1′-5″		1 151	CTD	1								11_6 011
S513	SERIES OF 68	TO 39'-7''		1,454	STR	1							Ι'	0′-6.8′′
	00	39-11	<del>'  </del>		+	+	-+		1	+	-	$\dashv$	+	
	·	SUBTO	JBTOTAL	65,486					L					
					BRIDO	GE PA	4RAPE	TS						
LP501	510	7′-4′′	4"	3,901	23				3′-0′′			0'-	-2.8"	
LP502	144	4'-6''		676	STR				L					
LP503	2	3′-5″		7	STR									
LP504	2	3′-10″		8	STR		-			$\perp$			$\bot$	
LP505	2	15'-2"		32	STR				-	+	_	-	$\rightarrow$	
LP506 LP507	2 2	15′-3′′ 15′-7′′		32 33	STR STR		-+		1	+	+	+	-+	
LP508	2	15'-8"		33	STR		-+		1			-	+	
LP509	16	10'-0''		167	STR								-	
LP510	16	5′-8′′	8''	95	25	1'-	-10''	2′-5′′	1′-5′′	0′-1.5	′′ 0′-5	5″		
LP511	16	5′-8′′		95	STR									
LP512	8	26'-0''		217	STR				1			-	$-\!$	
LP513	8	26'-6"		221	STR		-+		1	_		-	$\perp$	
LP514	32	40'-0"		1,335	STR				1	+	_	$-\!\!\!\!\!+\!\!\!\!\!\!-$	-+	
LP515	8	33′-7′′	1	280	STR	+	-+		1	-	-	$-\vdash$	-+	
LP601	72	4'-6"	6"	487	STR	+	-+		1	+	+	-	-+	
LP601 LP602	510	2'-5"		1,851	1		'-0''	1'-6''	1	+		-	+	
LP603	510	3'-2"		2,426	28			1'-0''	0'-11''	1			-+	
LP604	1	3′-5″	5"	5	STR			. •	T "			$\neg$	-+	
LP605	1	3′-10′′	0"	6	STR				L	1				
	8	3′-11′′	11''		1			3′-0′′						
LP606	SERIES OF	TO	)	631	1	1'-	'-0''	TO						0'-1''
	12	4′-10′′						3′-11′′	1			$\perp$		
LP607	1	15'-2"		23	STR		-+		1	_		-	$\perp$	
LP608 LP609	1	15′-3′′ 15′-7′′		23 23	STR STR	+	-+		1	+	+	-	-+	
LP609 LP610	1	15'-7''		23	STR	+			1	+		-	+	
LP610 LP611	32	4'-0"		192	1	1'-	'-0''	3′-1′′	1	+	_	$\dashv$	+	
	1				† <i>'</i>	1	- 1		1			$\neg$	-	
		SUBTO	JBTOTAL	12,820		$oldsymbol{\perp}$								
							$\Box$					$\perp$	$\bot$	
	SUBSTRUCTUR	TURE		5,851	_	+	-+		-	_		$-\!$	$\rightarrow$	
	SUPERSTRUCTU	TAL		85,780		+			1	+	_	_	$-\!\!+$	
	GRAND TOTA	I AL		91,630	+	+	-+		1	+	+	+	+	

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

REINFORCING STEEL LIST BRIDGE NO. MAR-23-0185 L&R OVER SR 98

DESIGN AGENCY
OHIO DEPARTMENT OF
TRANSPORTATION DISTRICT 6

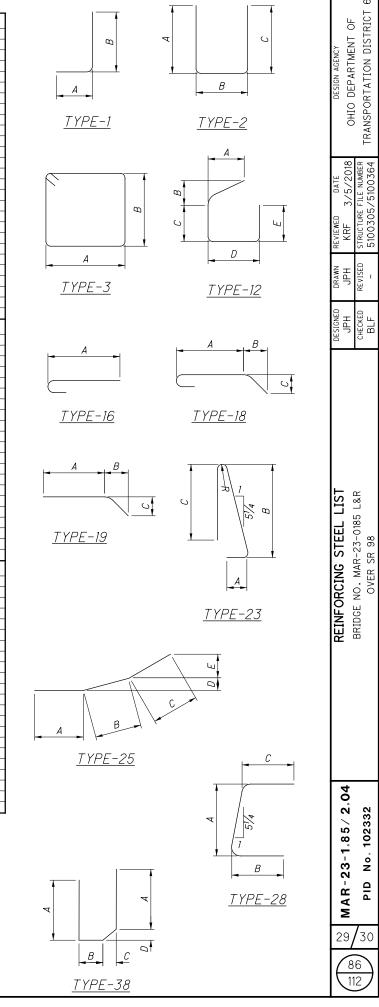
RIGHT BRIDGE FORWARD WINGWALLS (7 & 8) NUMBER DIMENSIONS RFW501 MARKLENGTH WEIGHT 13'-2'' 7'-5'' STR STR RFW502 TOTAL R INCΑ D Ε 70 RFW503 9'-0" STR REW504 25 235 RIGHT BRIDGE REAR ABUTMENT 2 4'-6" 1'-2" 3 1'-2" 2'-7" 3 1'-2" 3'-9" 37 3'-0" 3'-0" RFW505 9'-11'' 176 67 4'-6'' REW506 8'-1'' Α RRA501 SERIES OF TO TO 0'-0.2' RFW507 10′-5′′ 22 RFW508 9'-8'' 20 2'-0" 6'-5" 3′-10′ RFW509 19′-3′′ 120 STR TYPE-1 STR RRA502 SERIES OF TO 110 2'-8" TO 0'-0.3" RFW510 14'-1'' 29 6'-9'' 4'-2" RFW511 12'-9'' 106 STR RRA503 3′-10′′ 5′-11′′ 49 38 0′-7′′ 0'-7" 0'-6" 8'-6" RFW512 7′-3′′ 3′-10″ RFW513 6'-4'' 3′-8′′ RRA504 3′-6′′ 119 3′-0′′ RFW514 0'-9" RRA505 6'-9'' 14 3′-10′′ 1 3'-10" RFW515 13'-8'' STR RRA506 6'-0" 2'-3" 29 1.3 31′-0′′ 259 STR 12'-3'' 26 STR RFW516 RRA507 STR STR RFW517 11'-2'' 17′-3′′ 54 32'-4" STR RRA801 691 RFW518 STR STR RFW519 16'-0'' 50 13 SUBTOTAL 1242 RFW520 12'-4'' STR STR STR RIGHT BRIDGE REAR WINGWALLS (3 & 4) RFW521 11'-1'' 12 RFW522 10′-8′′ Α 45 RRW501 120 2'-9" 3'-9" RFW523 9′-5′′ 39 STR RRW502 10'-8'' 78 7′-11′′ RFW524 STR STR STR 7′-2′′ 67 TYPE-3 RRW503 9'-7" 20 3'-0'' RFW525 37 3′-0′′ 2'-0'' 8'-9'' 219 167 RRW504 2 3 3 37 STR STR 1'-8'' 2'-7'' 9′-5″ 4'-9'' 4'-9" RRW505 SUBTOTAL 1573 1'-8'' 9'-1'' 66 RRW506 RIGHT BRIDGE REAR DIAPHRAGM 5'-4'' 1'-2'' 3'-0'' RRW507 2'-2" D801 18 3′-8′′ 8'-10'' 5′-10′′ *592* 1'-0'' RRW508 18 38 1'-0'' RRW509 16'-0'' 100 RRD501 3′-7′′ 13′-5′′ 378 RRW510 11′-3′′ 23 2'-11" 27 STR 38 1 19 249 3'-2'' RRW511 10′-2′′ 85 RRD502 8'-10" 2′-9′′ 3'-6'' 2'-10'' 1′-0′′ 0'-2" RRW512 5′-11′′ 49 2'-6" 0'-2'' RRD503 13′-5′′ 14 2′-11′′ 2'-9" RRW513 5′-11′′ 123 3′-3″ RRD504 8'-1'' 2'-9'' RRW514 3′-10′′ 2'-11'' 0′-9′′ 0′-7′′ RRD505 15′-11′′ 2'-11'' 4'-9'' STR STR 2'-9" RRW515 13'-6'' RRD506 9'-4'' 10 2'-9'' 4'-1'' 12'-5" 7'-7" 3'-0" RRW516 12'-6" RRD507 2'-11'' RRW517 11′-5′′ STR RRD508 2'-9'' 2'-4" 17'-8" 55 STR RRD509 10'-1'' RRW518 STR STR 16'-4'' RRD510 6'-9'' 2'-9'' 1′-6′′ 2'-9" RRW519 RRW520 12'-8'' RRD511 9'-9" RRW521 11′-6′′ STR RRD512 2'-11" 12 11'-0'' 46 STR RRW522 STR STR 37 RRD801 6'-8'' 1′-7′′ 1'-6'' 9'-10'' 7'-7'' 142 3′-8′′ RRW523 41 30′-7′′ 1307 STR RRD802 RRW524 20 19'-7'' 314 STR 9'-7" 2'-11'' RRD803 3'-0" RRW525 2'-1" 31'-0'' 497 STR RRD804 TYPE-19 SUBTOTAL 1473 SUBTOTAL 3618 RIGHT BRIDGE FORWARD ABUTMENT RIGHT BRIDGE FORWARD DIAPHRAGM 10'-11'' 4'-3" RFA501 SERIES OF TO 187 TO 0'-0.5" D801 38 5′-10′′ 592 18 3'-8" 1'-0" 1'-0'' 11'-6'' 4'-5" 10′-5″ RFD501 4'-0' 2'-11'' 3'-8'' TO TO 0'-0.6" RFD502 RFA502 SERIES OF 192 2'-8" 9'-0" 253 14 3'-2" 11'-3'' 13'-7'' 2'-10'' 4'-5" RFD503 2 3 3'-2'' 2'-10'' RFA503 4'-3" RFD504 9′-1′′ 3'-0" 3'-2" 4'-10" 8'-11'' 7'-9'' 19 4'-3" RFA504 3'-8" 15′-10′′ 4'-10'' RFD505 1 16 4'-1'' RFD506 10'-2'' 2 3 2 3 RF4505 7′-3′′ 4'-0'' 15 3′-5′′ 12'-11'' 8'-10'' 3′-4′′ RFD507 2'-10'' RF4506 6'-2'' 13 2'-3" 4'-0'' 2'-9" 2'-3" 1'-6" 3'-7" RFD508 RFA507 31'-0'' 259 STR 3′-2′′ 2'-10'' 3'-2'' RFD509 10'-9'' 7'-7'' RFA801 STR RFD510 32'-4" 691 9′-7′′ RFD511 20 0'-11'' SUBTOTAL 1391 2'-2" RFD512 7′-0′′ 15 2 2'-2" 2'-11'' STR RFD513 3′-6′′ RFD801 1′-7′′ 6'-8" 3′-8′′ 1'-6'' TYPE-25 30′-11′′ RFD802 1321 STR RFD803 20'-0" 320 502 STR STR RFD804 SUBTOTAL 3654

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 



TYPE-37 В

No. 102332 PID 29/30

													<del></del> -	9
			RI		DGE DEC	CK						I	<del>†</del>     <del> </del>	
5401	240	40'-0''	6413	STR										DESIGN AGENCY OHIO DEPARTMENT OF TRANSPORTATION DISTRICT
S402 S403	60 192	33'-7" 2'-0"	1346 257	STR	0'-8''	1′-6′′	1	1				$ \mathcal{B} $	7 0	TN TSI
S404	192	2'-3"	289	19	1'-6"	0'-8"	0'-4"		<del>                                     </del>					WE WE
S405	192	1′-10′′	235	1	0'-8''	1'-3''							<u> </u>	AGE ION
S406	192	2'-0''	<i>2</i> 57	19	1′-3′′	0′-8′′	0'-4''					.	В	P A T I
												_ A _	<u> </u>	DE OE
S501	1 SERIES OF	2'-4'' TO	1491	16	1′-6″ TO				1 1	0′-6.9				0 0
3507	67	40'-4''	1431	1 10	39'-6"				1 1	0 -0.3		<u>TYPE-1</u>	<u>TYPE-2</u>	NS I
	1	1'-6''			1	1						<del></del>	<u> </u>	RA
S502	SERIES OF	ТО	1450	STR					1 1	0'-6.9			4	⊢
C507	67 319	39'-6'' 20'-10''	0070	10	20'-0"								<del>- A</del> -	18 18 19
S503 S504	319	20'-10"	6932 6654	16 STR	20'-0"	1			<del>                                     </del>					201 201 UMBE
S505	2	24'-4"	51	16	23'-6"							<u>``</u>   [	~	DATI 5/2 100
S506	2	23′-6″	49	STR										25 € 5
S507	2	24'-10"	52	16	24'-0''							$  \Theta  $	<b>†</b>	05,
S508 S509	2 315	24'-0'' 26'-9''	50 8789	STR 16	25′-11′′	1	1	-	<del>                                     </del>					ST CTU
S510	315	25'-11"	8515	STR	20 11				<del>                                     </del>			\	• • •	KRF STRUCTURE 15100305,
S511	472	40'-0''	19692	STR								A	D .	8 8 9
	1	2′-3″		l	1′-5′′				1 1			→ A →	<del></del>	≥ + □
S512	SERIES OF 68	TO 40′-5″	1513	16	TO 39'-7''				1 1	0'-6.8		TVDC 7		DRAWN JPH REVISED
.	1	1′-5″			1 33 /	1						<u>TYPE-3</u>	<u>TYPE-12</u>	
S513	SERIES OF	TO	1454	STR					1 1	0'-6.8				
	68	39′-7″												N
I -		SUBTOTAL	65486											DESIGNET JPH CHECKED BLF
l <del>                                    </del>		SUBTUTAL		IT POTO	<u>I</u> SE PARP	I DETC			<u> </u>			Δ	A B	
RP501	512	7′-4′′	3916		L CARE	3'-3"	3′-0′′	1				<del>-                                    </del>		
RP502	72	4'-6"	338	23 STR	1 0 -11	1 3 -3	1 3 -0							
RP503	2	4'-2"	9	STR										
RP504	2	4′-7′′	10	STR								_	<u> </u>	
RP505 RP506	2 2	15'-2'' 15'-3''	32 32	STR STR		1			-			<u>TYPE-16</u>	<u>TYPE-18</u>	
RP507	2	15'-8''	33	STR			1		<del>                                     </del>			<u> </u>	<u> </u>	
RP508	2	15′-9′′	33	STR										
RP509	16	10'-0''	167	STR										
RP510 RP511	16 16	5′-8′′ 5′-8′′	95 95	25 STR	1′-10′′	2'-5"	1′-5′′	0'-1.5"	0′-5′′			$A = B_{-1}$	<del>                                     </del>	
RP512	8	25'-10"	216	STR										
RP513	8	26′-3″	219	STR									-     <del>\</del> \ <u>/</u>	
RP514	32	40′-0′′	1335	STR								$\setminus \mid \circ \mid$	2 \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	l≌ <sup>∞</sup> l
RP515	8	34′-1′′	284	STR		1			-			\	-	85
RP601	74	4'-6''	500	STR					<del>                                     </del>			<u>TYPE-19</u>	<b>\</b>   \ \	
RP602	512	2'-5"	1859	1	1'-0'' 1'-6''	1′-6′′						<u> </u>		23. 98.
RP603	512	3'-2"	2435	28	1'-6''	1'-0''	0'-11''						<del></del>	S 4 8
RP604 RP605	1	4'-2'' 4'-7''	<u>6</u> 7			-			<del></del>					S ≥ H
N-003	8	3'-11"				3′-0″							<del>-</del> A	
RP606	SERIES OF	ТО	79	1	1'-0''	TO			1 1	0'-1''				16 Z 1
<u> </u>	12	4′-10′′				3′-11′′							<u>TYPE-23</u>	<b> 吟                                    </b>
RP607 RP608	1	15'-2'' 15'-9''	23 24	STR STR									<del></del>	
RP609	1	15'-8''	24	STR		1								REINFORCING STEEL LIST BRIDGE NO. MAR-23-0185 L&R OVER SR 98
RP610	1	15′-9′′	24	STR										
RP611	32	4'-0''	192	1	1'-0''	3'-1''	-						ш	
	1	SUBTOTAL	11983			1			<del>                                     </del>					
		33737712	7,000	1		1							C	
													6	
		SUBSTRUCTURE	5679	_			1					1   B   `		
SI		SUBSTRUCTURE SUPERSTRUCTURE GRAND TOTAL	84742 90421	-		1			<del>                                     </del>			A \		
		CHAID TOTAL	00 121		-	-	-	-				TVDE 25	0	
3												<u>TYPE-25</u>	<del> </del>	
														<u> </u>
													<u> </u>	1.85 / 2.04
5														ا ۾ ٿي ا
: <b> </b>											, A ,		5/4	
														102332
													<b>1</b>	= =
												†		23-1 No.
												_	В	2 z
												۲		MAR-23-
												\ \ \ <del>\</del>	<u>TYPE-28</u>	A
														<u>s</u>
5											B A	B $C$		
[														30/30
[											<u>TYPE-37</u>	<u>TYPE-38</u>		
3											<u>· · · · · · · · · · · · · · · · · · · </u>	<u> </u>		87

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

# NOTES 23-0204 L TURE NO. MAR-23-STRUCT BRIDGE NO. I OVER Q

OF RICT

OHIO DEPARTMENT TRANSPORTATION DIST

# 9 ญ่ 102332 1.85, ŝ PID

# STANDARD DRAWINGS AND SUPPLEMENTAL *SPECIFICATIONS*

REFER TO	THE FOLLOWING STANDARD	BRIDGE DRAWING(S)
AS-1-15	<i>DATED/REVISED</i>	07-17-15
15-2-15	DATED / PEVICED	01-18-10

*DATED/REVISED* 01-18-19 GSD-1-19 DATED/REVISED 01-18-19 PCB-91 01-18-13 DATED/REVISED SBR-1-13 DATED/REVISED 07-20-18 SICD-1-96 DATED/REVISED 07-18-14 SICD-2-14 DATED/REVISED 07-18-14

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATION(S): SS869 DATED/REVISED 10-17-14

# DESIGN SPECIFICATIONS

DESIGN SPECIFICATIONS: THIS STRUCTURE CONFORMS TO THE 8TH EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2017 AND THE ODOT BRIDGE DESIGN MANUAL, 2019.

# DESIGN LOADING

DESIGN LOADING: HL-93 (SUPERSTRUCTURE ONLY)

FUTURE WEARING SURFACE (FWS) OF 0.060 KIPS/SQ.FT.

# DESIGN DATA

CONCRETE CLASS QC2 - COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE)

CONCRETE CLASS QC1 - COMPRESSIVE STRENGTH 4.0 KSI (SUBSTRUCTURE)

REINFORCING STEEL - MINIMUM YIELD STRENGTH 60 KSI

STRUCTURAL STEEL - ASTM A709 GRADE 50W - YIELD STRENGTH 50 KSI

# DECK PROTECTION METHOD

EPOXY COATED REINFORCING STEEL

2.5" CONCRETE COVER

# MONOLITHIC WEARING SURFACE

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1" THICK.

# ASBESTOS NOTIFICATION

AN ASBESTOS SURVEY OF THE BRIDGE WAS CONDUCTED BY A CERTIFIED ASBESTOS HAZARD EVALUATION SPECIALIST. THE SURVEY DETERMINED THAT NO ASBESTOS IS PRESENT AT THE

A COPY OF THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) NOTIFICATION OF DEMOLITION AND RENOVATION FORMS, PARTIALLY COMPLETED AND SIGNED BY THE BRIDGE OWNER, WILL BE PROVIDED TO THE SUCCESSFUL BIDDER. THE CONTRACTOR SHALL COMPLETE THE FORM AND SUBMIT IT TO:

OHIO EPA, CDO 50 WEST TOWN ST., SUITE 700 COLUMBUS, OHIO 43215 KELLY TOTH, APC MANAGER (614) 728-3778 FAX: (614) 728-3898

AT LEAST TEN (10) WORKING DAYS PRIOR TO THE START OF ANY DEMOLITION AND/OR REHABILITATION. THE CONTRACTOR SHALL PROVIDE A COPY OF THE COMPLETED FORM TO THE

INFORMATION REQUIRED ON THE FORM WILL INCLUDE: 1) THE CONTRACTOR'S NAME AND ADDRESS, 2) THE SCHEDULED DATES FOR THE START AND COMPLETION OF THE BRIDGE REMOVAL, AND 3) A DESCRIPTION OF THE PLANNED DEMOLITION WORK AND THE METHOD(S) TO BE USED.

THE CONTRACTOR SHALL FURNISH ALL FEES, LABOR, AND MATERIAL NECESSARY TO COMPLETE AND SÚBMIT THE OEPA NOTIFICATION FORM.

# WATERWAY/WETLAND PROTECTION

THE CONTRACTOR SHALL NOT PLACE TEMPORARY OR PERMANENT FILL IN WETLANDS OR BELOW THE ORDINARY HIGH WATER MARK OF ANY WATERWAY DURING CONSTRUCTION OF THIS PROJECT, INCLUDING SCAFFOLDING OR BRACING. THE CONTRACTOR SHALL NOT PLACE/STAGE EQUIPMENT BELOW THE ORDINARY HIGH WATER MARK. IF DEBRIS ENTERS THE WATERWAY DURING CONSTRUCTION, THE CONTRACTOR SHALL REMOVE THE DEBRIS IMMEDIATELY UTILIZING HAND REMOVALS OR EQUIPMENT STAGED ABOVE THE ORDINARY HIGH WATER

# DECK PLACEMENT DESIGN ASSUMPTIONS

DECK PLACEMENT DESIGN ASSUMPTIONS: THE FOLLOWING
ASSUMPTIONS OF THE CONSTRUCTION MEANS AND METHODS
WERE MADE FOR THE ANALYSIS AND DESIGN OF THE
SUPERSTRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR
THE FALSEWORK SUPPORT SYSTEM WITH THESE PARAMETERS
AND WILL ASSUME RESPONSIBILITY FOR SUPERSTRUCTURE ANALYSIS FOR DEVIATION FROM THESE DESIGN ASSUMPTIONS.

AN EIGHT WHEEL FINISHING MACHINE WITH A MAXIMUM WHEEL LOAD OF 2.40 KIPS FOR THE LEFT BRIDGE AND 2.28 KIPS FOR THE RIGHT BRIDGE, A MINIMUM OUT-TO-OUT WHEEL SPACING AT EACH END OF THE MACHINE OF 103".

A MAXIMUM SPACING OF OVERHANG FALSEWORK BRACKETS OF A MAXIMUM DISTANCE FROM THE CENTERLINE OF THE FASCIA GIRDER TO THE FACE OF THE SAFETY HANDRAIL OF 65".

# EXISTING STRUCTURE VERIFICATION

EXISTING STRUCTURE VERIFICATION: DETAILS AND DIMENSIONS SHOWN ON THESE PLANS PERTAINING TO THE EXISTING STRUCTURE HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND THE PROPOSED WORK BUT THEY SHALL BE CONSIDERED TENTATIVE AND APPROXIMATE. THE CONTRACTOR IS REFERRED TO CMS SECTIONS 102.05, 105.02, AND 513.04. BASE CONTRACT BID PRICES UPON A PREBID EXAMINATION OF THE EXISTING STRUCTURE. HOWEVER. THE DEPARTMENT WILL PAY FOR ALL PROJECT WORK BASED UPON ACTUAL DETAILS AND DIMENSIONS WHICH HAVE BEEN VERIFIED IN THE FIELD.

# ITEM 202 - PORTIONS OF STRUCTURE REMOVED. AS PER PLAN

DESCRIPTION: THIS WORK CONSISTS OF THE REMOVAL OF THE ENTIRE EXISTING SUPERSTRUCTURE. THIS WORK SHALL INCLUDE THE REMOVAL OF ALL EXISTING CONCRETE DECK, RAILING, BEAMS, INTERMEDIATE CROSSFRAMES, END CROSSFRAMES, EXPANSION JOINTS, ABUTMENT AND PIER BEARING ASSEMBLIES, AND ALL OTHER INDIVIDUAL COMPONENTS OF THE ENTIRE EXISTING SUPERSTRUCTURE. THIS WORK ALSO CONSISTS OF THE PARTIAL REMOVAL OF THE SUBSTRUCTURE AS DETAILED IN THE PLAN.

DURING THE BEARING REMOVAL PROCESS, DRILL OUT ALL EXISTING ANCHOR BARS, FILL THE HOLES WITH NONSHRINK NONMETALLIC GROUT AS PER C&MS 510. AND ENSURE THAT BEARING SEAT AREAS HAVE A PURE AND LEVEL SURFACE TO REST ON. THE CONTRACTOR SHALL FOLLOW C&MS 516.07 TO ACCOMPLISH THIS SEAT PREPARATION WORK FOR NEW BFARINGS.

PROTECTION OF TRAFFIC: PRIOR TO DEMOLITION OF ANY PORTIONS OF THE EXISTING SUPERSTRUCTURE, THE CONTRACTOR SHALL SUBMIT PLANS FOR THE PROTECTION OF TRAFFIC (VEHICULAR, PEDESTRIAN, BOAT, ETC.) AS PER CMS

REMOVAL METHODS: REMOVAL METHODS OVER STRUCTURAL MEMBERS SHALL ENSURE ADEQUATE DEPTH CONTROL AND PREVENT NICKING OR GOUGING THE PRIMARY STRUCTURAL MEMBERS WHILE STILL IN USE DURING THE REMOVAL

MEASUREMENT & PAYMENT: THE DEPARTMENT WILL MEASURE THE QUANTITY OF REMOVALS ON A LUMP SUM BASIS. THE DEPARTMENT WILL PAY FOR THE ACCEPTED QUANTITIES OF REMOVALS AT THE CONTRACT PRICE FOR ITEM 202, PORTIONS OF STRUCTURE REMOVED. AS PER PLAN.

# CUT LINE CONSTRUCTION JOINT PREPARATION

SAW CUT BOUNDARIES OF PROPOSED CONCRETE REMOVALS I INCH DEEP. REMOVE CONCRETE TO A ROUGH SURFACE. LEAVE THE EXISTING REINFORCING STEEL, IF REQUIRED IN THE PLANS, IN PLACE. INSTALL DOWEL BARS IF SPECIFIED. PRIOR TO CÓNCRETE PLACEMENT ABRASIVELY CLEAN JOINT SURFACES AND EXISTING EXPOSED REINFORCEMENT TO REMOVE LOOSE AND DISINTEGRATED CONCRETE AND LOOSE RUST. THOROUGHLY CLEAN THE JOINT SURFACE AND EXPOSED REINFORCEMENT OF ALL DIRT, DUST, RUST, OR OTHER FOREIGN MATERIAL BY THE USE OF WATER, AIR UNDER PRESSURE, OR OTHER METHODS THAT PRODUCE SATISFACTORY RESULTS. EXISTING REINFORCING STEEL DOES NOT HAVE TO HAVE A BRIGHT STEEL FINISH, BUT REMOVE ALL PACK AND LOOSE RUST. THOROUGHLY DRENCH EXISTING CONCRETE SURFACES WITH CLEAN WATER AND ALLOW TO DRY TO A DAMP CONDITION BEFORE PLACING CONCRETE.

# SUBSTRUCTURE CONCRETE REMOVAL

ALL CONCRETE REMOVED AS DETAILED IN THE PLANS SHALL BE REMOVED BY MEANS OF CUTTING AND OF APPROVED PNEUMATIC HAMMERS EMPLOYING POINTED AND BLUNT CHISEL TOOLS. HYDRAULIC HOE-RAM TYPE HAMMERS WILL NOT BE PERMITTED. THE WEIGHT OF THE HAMMERS SHALL NOT BE MORE THAN 35 POUNDS FOR REMOVAL WITHIN 18 INCHES OF PORTIONS TO BE PRESERVED. OUTSIDE THE 18 INCH LIMIT, THE CONTRACTOR MAY USE HAMMERS NOT EXCEEDING 90 POUNDS UPON THE APPROVAL OF THE ENGINEER. DO NOT PLACE PNEUMATIC HAMMERS IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE REBUILT STRUCTURE.

# ITEM 509 - EPOXY COATED REINFORCING STEEL, AS PER PLAN

ALL PROVISIONS OF C&MS 509 SHALL APPLY FOR REINFORCING STEEL DETAILED IN THIS PLAN WITH THE FOLLOWING PROVISIONS.

PROVIDE EUSION BONDED FPOXY COATED REINFORCING STEEL CONFORMING TO ASTM A934 "STANDARD SPECIFICATION FOR EPOXY COATED PREFABRICATED STEEL REINFORCING BARS."

ENSURE THAT STEEL REINFORCING BARS TO BE COATED CONFORM TO 709.01, 709.03, 709.05, AND ARE FREE OF OIL, GREASE, OR PÁINT.

ENSURE THAT THE COATING MATERIAL MEETS THE REQUIREMENTS LISTED IN ANNEX AT AND IS A COLOR THAT FACILITATES INSPECTION OF THE INSTALLED BAR.

# REPORT OF TEST RESULTS IS REQUIRED.

SAMPLES OF FUSION BONDED EPOXY COATED REINFORCING STEEL ARE REQUIRED. PERFORM TEST, INSPECTION, AND SAMPLING AT A SITE SPECIFIED BY THE ENGINEER. SAMPLING FOR TESTING REQUIRES THREE 30 INCH (IM) SAMPLES FOR EACH BAR SIZE, FOR EACH COATING LOT, AND FOR EACH HEAT OF STEEL REINFORCING BARS.

ITEM 526 - REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN ITEM 526 - TYPE C INSTALLATION, AS PER

THE CONTRACTOR SHALL USE FUSION BONDED EPOXY COATED REINFORCING STEEL FOLLOWING THE GUIDELINES AND SPECIFICATIONS FOR ITEM 509 - REINFORCING STEEL, AS

THE PAYMENT FOR FUSION BONDED EPOXY COATED REINFORCING STEEL SHALL BE INCLUDED WITH ITEM 526 -TYPE C INSTALLATION, AS PER PLAN.

ITEM 516 - 2" DEEP JOINT SEALER, AS PER PLAN

UPON COMPLETION OF THE PROPOSED APPROACH SLAB THE CONTRACTOR SHALL SAW CUT ALONG THE APPROACH SLAB AND BRIDGE LIMIT, AS DETAILED IN THE PLANS, AN AREA 1" WIDE BY 2" DEEP AND FILL THIS AREA WITH HOT APPLIED JOINT SEALER 705.04.

# DECK SLAB CONCRETE QUANTITY

THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS BASED ON THE CONSTANT DECK SLAB THICKNESS, AS SHOWN, PLUS THE QUANTITY OF CONCRETE THAT FORMS EACH BEAM THE QUANTITY OF CONCRETE THAT FORMS EACH BEAM HAUNCH. THE ESTIMATE ASSUMES A CONSTANT HAUNCH HEIGHT OF 2" AND A HAUNCH WIDTH EQUAL TO THE TOP FLANGE WIDTH, DEVIATE FROM THIS HAUNCH THICKNESS AS NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED

THE HAUNCH THICKNESS WAS MEASURED AT THE CENTERLINE OF THE BEAM, FROM THE SURFACE OF THE DECK TO THE BOTTOM OF THE TOP FLANGE MINUS THE DECK SLAB THICKNESS. THE AREA OF ALL EMBEDDED STEEL PLATES HAS BEEN DEDUCTED FROM THE HAUNCH QUANTITY IN ACCORDANCE

# ITEM 513 - STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED HOLES

THIS ITEM SHALL CONSIST OF FIELD LOCATIONS AND FIELD DRILLING OF 2" DIAMETER HOLES FOR #8 REINFORCING STEEL AS PER STD. DWG. SICD-1-96 INTO THE BEAM ENDS. THE CONTRACTOR SHALL CLEARLY MARK THE LOCATIONS AND HAVE THE ENGINEER'S APPROVAL PRIOR TO DRILLING. FLAME CUTTING OF THE HOLES WILL NOT BE PERMITTED.

MEASUREMENT: THE DEPARTMENT WILL PAY FOR EACH 2" DIA. HOLE DRILLED.

ALL MATERIALS, LABOR, INCIDENTALS TO PERFORM THE ABOVE WORK SHALL BE INCLUDED IN THE BID PRICE FOR ITEM 513 - STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED

# WELDED ATTACHMENTS

WELD ATTACHMENT OF SUPPORTS FOR CONCRETE DECK FINISHING MACHINE TO AREAS OF THE FASCIA STRINGER FLANGES DESIGNATED "COMPRESSION". DO NOT WELD ATTACHMENTS TO AREAS DESIGNATED "TENSION". FILLET WELDS TO COMPRESSION FLANGES SHALL BE AT LEAST 1"
FROM EDGE OF FLANGE, BE NO MORE THAN 2" LONG, AND BE
AT LEAST 1/4" FOR THICKNESSES UP TO 34" OR 36" FOR
GREATER THAN 34" THICK.

# ITEM 519 - PATCHING OF CONCRETE STRUCTURE

THE FOLLOWING ITEMS HAVE BEEN PROVIDED AS CONTINGENCY QUANTITIES TO BE USED AS DIRECTED BY THE ENGEER AT THE NOTED LOCATIONS. THESE ITEMS HAVE BEEN CARRIED TO THE STRUCTURES SUMMARY.

MAR-23-02041

ITEM 519 - PATCHING CONCRETE STRUCTURE = 50 SF

MAR-23-0204R

ITEM 519 - PATCHING CONCRETE STRUCTURE = 50 SF

ITEM 519 - PATCHING OF CONCRETE STRUCTURE AS PER PLAN

ALL PROVISIONS OF 514 SHALL APPLY FOR PATCHING OF CONCRETE PIER CAP WITH THE FOLLOWING PROVISION. ANY TEMPORARY SUPPORT OF THE BEAM IS INCIDENTAL TO THE PATCHING ITEM. SEE SHEET 1/25 FOR PATCHING LOCATION.

23 MAR

# ABUTMENT DIAPHRAGM CONCRETE

PLACE THE DIAPHRAGM CONCRETE ENCASING THE STRUCTURAL MEMEBER ENDS WITH THE DECK CONCRETE OR AT LEAST 48 HOURS BEFORE PLACEMENT OF THE DECK CONCRETE. IF PLACED SEPARATELY, LOCATE A HORIZONTAL CONSTRUCTION JOINT IN THE DIAPHRAGM AS SHOWN ON SICD-1-96 FOR STEEL SUPERSTRUCTURES AND PLACE THE REMAINING DIAPHRAGM CONCRETE WITH THE DECK CONCRETE WITH THE DECK.

# STRUCTURAL STEEL QUANTITIES

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

THE STRUCTURAL STEEL QUANTITIES LISTED IN THE STRUCTURES SUMMARY INCLUDE BEAMS, INTERMEDIATE DIAPHRAGMS, AND SPLICE ASSEMBLIES TO CONSTRUCT THE BRIDGE AS DETAILED. THE TOTAL QUANTITIES ARE BASED UPON THE ASSUMPTION OF TWO (2) FIELD SPLICES PER BEAM. EACH SPLICE ASSEMBLY HAS AN ESTIMATED WEIGHT OF 512.43 LBS. EACH BEAM HAS A TOTAL OF FOUR (4) FIELD SPLICE LOCATIONS DETAILED IN THE PLANS. ALL SPLICE LOCATIONS ARE OPTIONAL. THE DEPARTMENT WILL PAY FOR THE ACTUAL QUANTITY OF STEEL PLACED.

DESIGN AGENCY
OHIO DEPARTMENT OF
TRANSPORTATION DISTRICT 6 STRUCTURE NOTES
BRIDGE NO. MAR-23-0204 L8
OVER QU QUA CREEK 2.04 MAR-23-1.85/ ° N

1 bfiris	
.2332.MAR-23-1.85\Design\S+ructures\WAR023.0204L&R\Sheets\102332.SQ101.dgn 102332.SQ101 9/17/2020 1:38:38 PM t	
23.	l

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

				ESTIMATED QUANTITIES (MAR-23-0204L SFN: 5100399)					
ITEM	EXTENSION	TOTAL 1/NHS/BR	UNIT	DESCRIPTION	ABUT.	PIERS	SUPER.	GEN.	SHEET #
202	11201		LS	PORTIONS OF STRUCTURE REMOVED, AS PER PLAN					3/25
202	22900	200	SY	APPROACH SLAB REMOVED				200	
202	23500	733	SY	WEARING COURSE REMOVED				733	
503	11100		LS	COFFERDAMS AND EXCAVATION BRACING					
503	21300		LS	UNCLASSIFIED EXCAVATION					
509	10001	79,127	LB	EPOXY COATED REINFORCING STEEL, AS PER PLAN	4,475		74,652		3/25
510	10000	186	EACH	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT	186				
511	21522	242	CY	CLASS QC2 CONCRETE WITH QC/QA, SUPERSTRUCTURE			242		
511	33501	2	EACH	SEMI-INTEGRAL DIAPHRAGM GUIDE, AS PER PLAN			2		10/25
511	34448	54	CY	CLASS QC2 CONCRETE, BRIDGE DECK (PARAPET)			54		
511	45710	72	CY	CLASS QCI CONCRETE, ABUTMENT	72				
512	10050	464	SY	SEALING OF CONCRETE SURFACES (NON-EPOXY)				464	
513	10260	124,066	LB	STRUCTURAL STEEL MEMBERS, LEVEL 3			124.066		
513	20000	3,765	EACH	WELDED STUD SHEAR CONNECTORS			3.756		
513	95030	36	EACH	STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED HOLES			36		3/25
516	10010	106	FT	ARMORLESS PREFORMED JOINT SEAL				106	
516	13600	2	SF .	1" PREFORMED EXPANSION JOINT FILLER				2	
516 516	13900	5	SF	2" PREFORMED EXPANSION JOINT FILLER				5	
516 516	14020 31011	136 99	FT FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL  2" DEEP JOINT SEALER, AS PER PLAN			00	136	7.00
516	31011	99	FI	Z" DEEP JOINT SEALER, AS PER PLAN			99		3/25
516	41100	24	EACH	1/8" PREFORMED BEARING PAD				24	
516	44100	12	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE) (11"x9 1/2"x2.7226")				12	
516	44100	12	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE) (13*x12*x2.7226*)				12	
518	21200	74	CY	POROUS BACKFILL WITH GEOTEXTILE FABRIC				74	
518	40000	139	FT	6" PERFORATED CORRUGATED PLASTIC PIPE				139	
518	40010	48	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS				48	
519	11100	50	SF	PATCHING OF CONCRETE STRUCTURE				50	3/25
526	25001	292	SY	REINFORCED CONCRETE APPROACH SLABS (T=15"). AS PER PLAN				292	3/25
526	90031	106	FT	TYPE C INSTALLATION, AS PER PLAN				106	3/25
611	99710	4	FACH	PRECAST REINFORCED CONCRETE OUTLET				4	

ITEM	EXTENSION	TOTAL 1/NHS/BR	UNIT	DESCRIPTION	ABUT.	PIERS	SUPER.	GEN.	SHEET
202	11201	II THIO DI	LS	PORTIONS OF STRUCTURE REMOVED, AS PER PLAN					3/2
202	22900	134	SY	APPROACH SLAB REMOVED				134	
202	23500	588	SY	WEARING COURSE REMOVED				588	
503	11100		LS	COFFERDAMS AND EXCAVATION BRACING				LS	-
503	21300		LS	UNCLASSIFIED EXCAVATION				LS	$\top$
509	10001	76,935	LB	EPOXY COATED REINFORCING STEEL, AS PER PLAN	3,961		72,974		3/2
000		70,000		·	3,001		12,011		572
510	10000	160	EACH	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT	160				=
511	21522	199	CY	CLASS QC2 CONCRETE WITH QC/QA, SUPERSTRUCTURE			199		+-
511	33501	2	EACH	SEMI-INTEGRAL DIAPHRAGM GUIDE, AS PER PLAN			2		10/
511	34448	54	CY	CLASS QC2 CONCRETE, BRIDGE DECK (PARAPET)			54		
511	45710	65	CY	CLASS QCI CONCRETE, ABUTMENT	65				
512	10050	455	SY	SEALING OF CONCRETE SURFACES (NON-EPOXY)				455	
513	10260	102,526	LB	STRUCTURAL STEEL MEMBERS, LEVEL 3			102,526		-
513	20000	3,130	EACH	WELDED STUD SHEAR CONNECTORS			3.130		+
513	95030	30	EACH	STRUCTURAL STEEL, MISC.: 2" DIA. FIELD DRILLED HOLES			30		3/.
									=
516	10010	106	FT	ARMORLESS PREFORMED JOINT SEAL				106	_
516	13600	2	SF	1" PREFORMED EXPANSION JOINT FILLER				2	-
516	13900	5	SF	2" PREFORMED EXPANSION JOINT FILLER				5	₩
516	14020	116	FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL				116	<del>  .</del>
516	31011	80	FT	2" DEEP JOINT SEALER, AS PER PLAN			80		3/
516	41100	20	EACH	1/8" PREFORMED BEARING PAD				20	
516	44100	10	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE) (11"x9 1/2"x2.7226")				12	
516	44100	10	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE) (13*x12*x2.7226*)				12	_
518	21200	65	CY	POROUS BACKFILL WITH GEOTEXTILE FABRIC				65	
518	40000	119	FT	6" PERFORATED CORRUGATED PLASTIC PIPE				119	
518	40010	48	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS				48	_
519	11100	50	SF	PATCHING CONCRETE STRUCTURE				50	3/
519	11101	9	SF	PATCHING CONCRETE STRUCTURE, AS PER PLAN		9			3/
526	25001	238	SY	REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN				238	3/
526	90031	86	FT	TYPE C INSTALLATION, AS PER PLAN				86	3/
611	99710	4	EACH	PRECAST REINFORCED CONCRETE OUTLET				4	+

# 1331

DESIGNED DRAWN REVIEWED DATE

BLF BLF KRF 7/3/2020

CHECKED REVISED STRUCTURE FILE NUMBER

CHECKED REVISED STRUCTURE FILE NUMBER

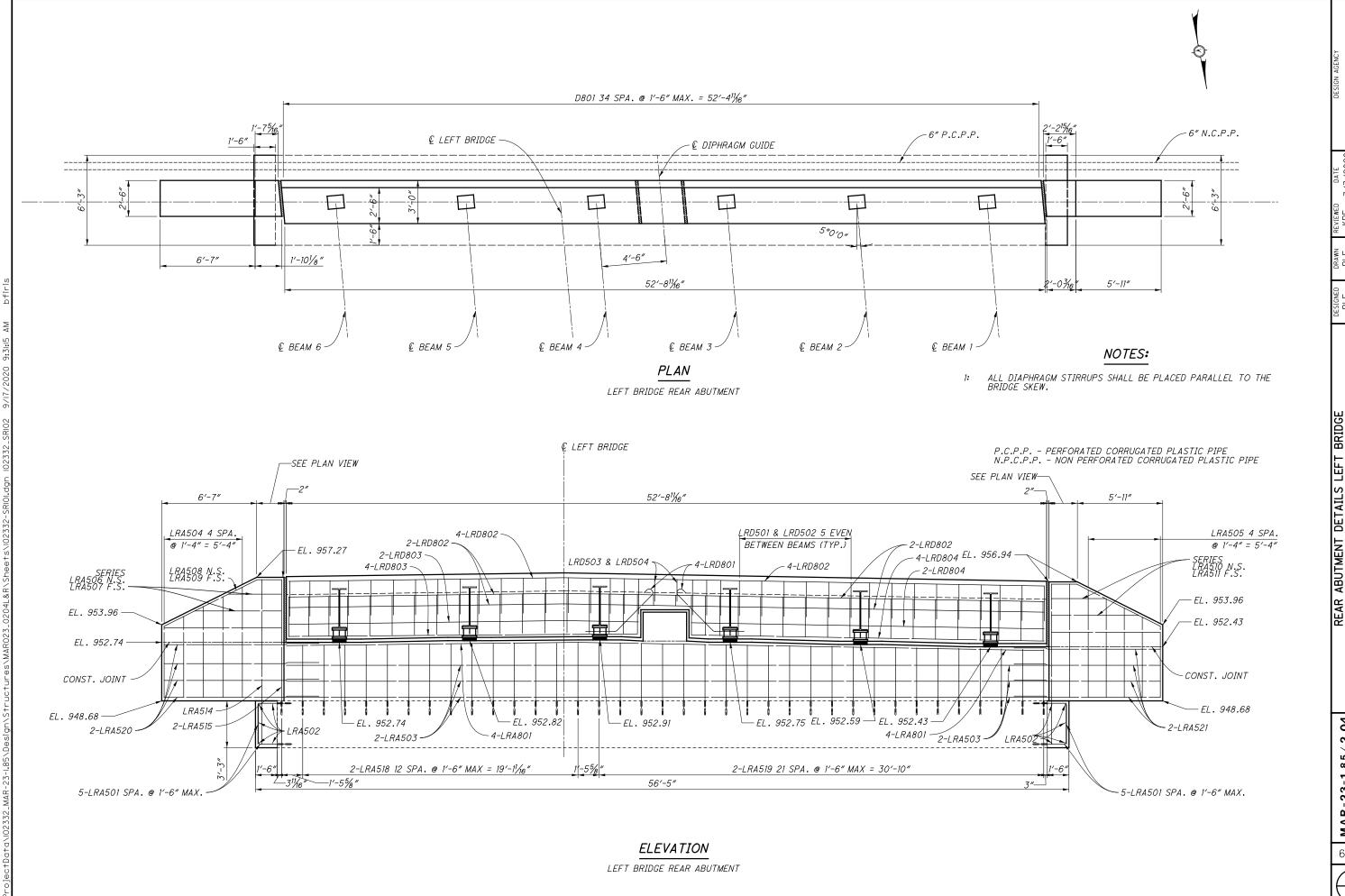
JPH - 5100399/5100429

ESTIMATED QUANTITIES
BRIDGE NO. MAR-23-0204 L&R
OVER QU QUA CREEK

MAR-23-1.85/2.04 PID No. 102332

**≥**4 /25

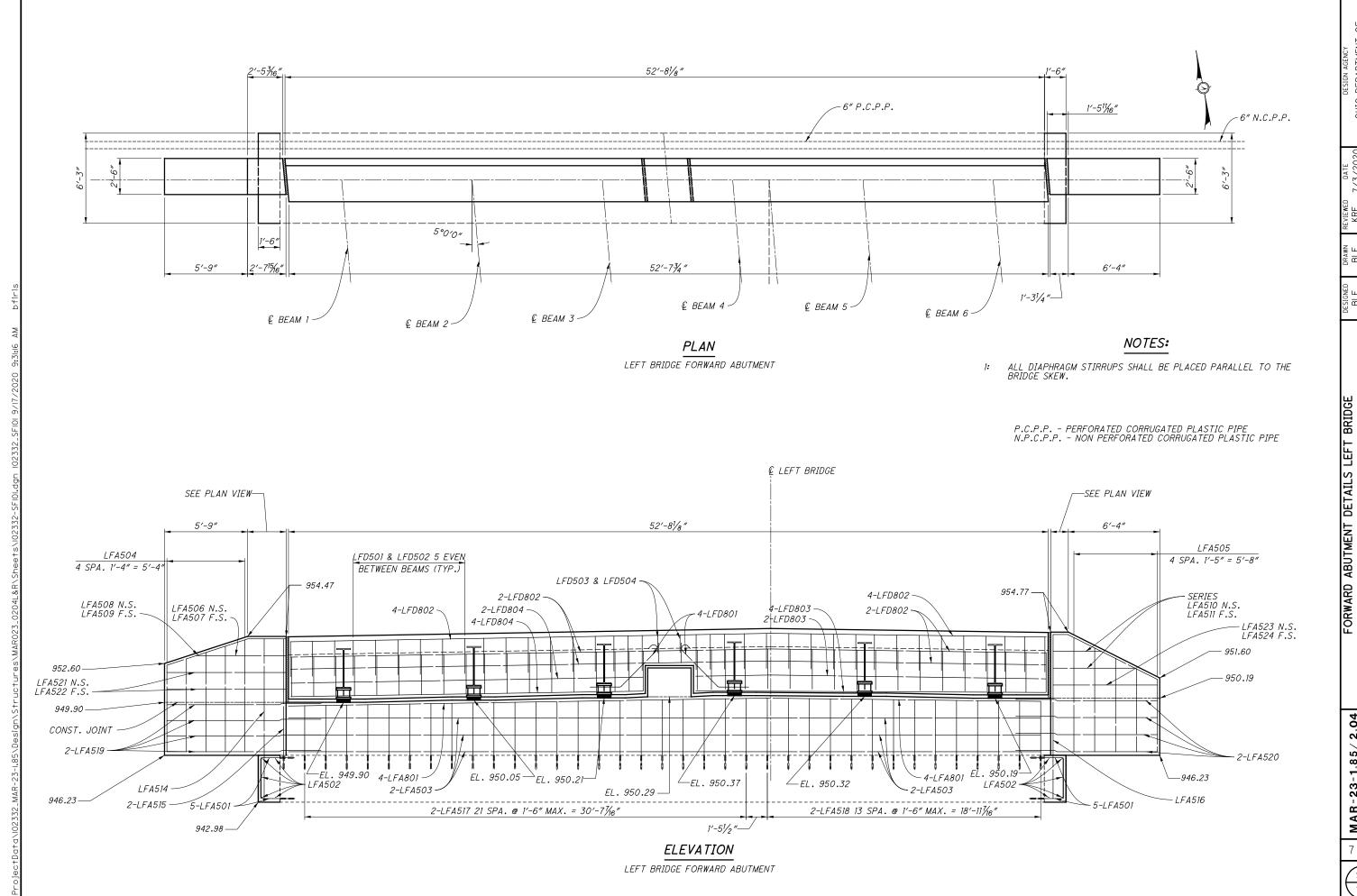




DESIGN AGENCY
OHIO DEPARTMENT OF
TRANSPORTATION DISTRICT

REAR ABUTMENT DETAILS LEFT BRIDGE BRIDGE NO. MAR-23-0204 L&R OVER OU QUA CREEK

MAR-23-1.85/2.04



 $\bigcirc$ 

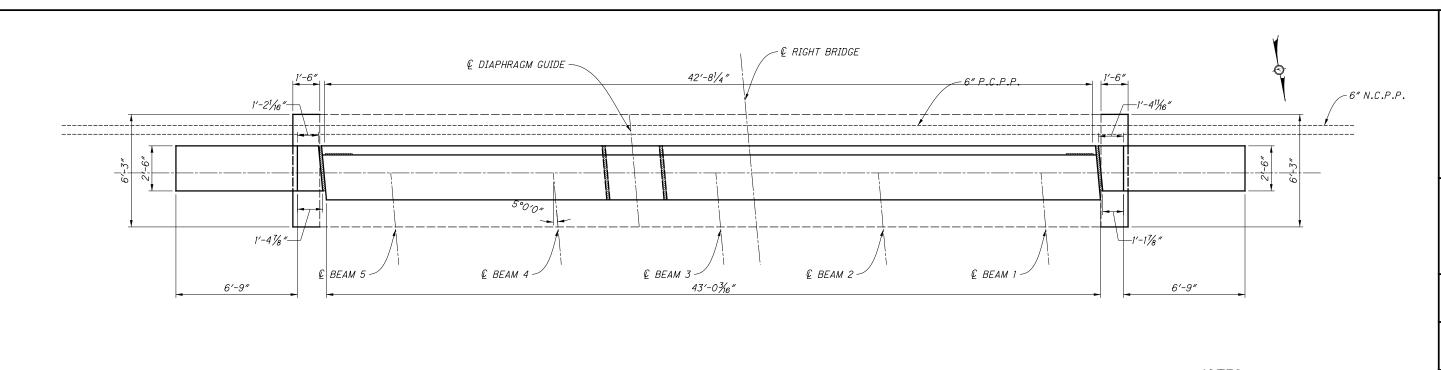
 $\bigcirc$ 

DESIGN AGENCY
OHIO DEPARTMENT OF
TRANSPORTATION DISTRICT

ABUTMENT DETAILS LEFT BRIDGE NO. MAR-23-0204 L&R OVER QU QUA CREEK

FORWARD

MAR-23-1.85/2.04 No. 102332



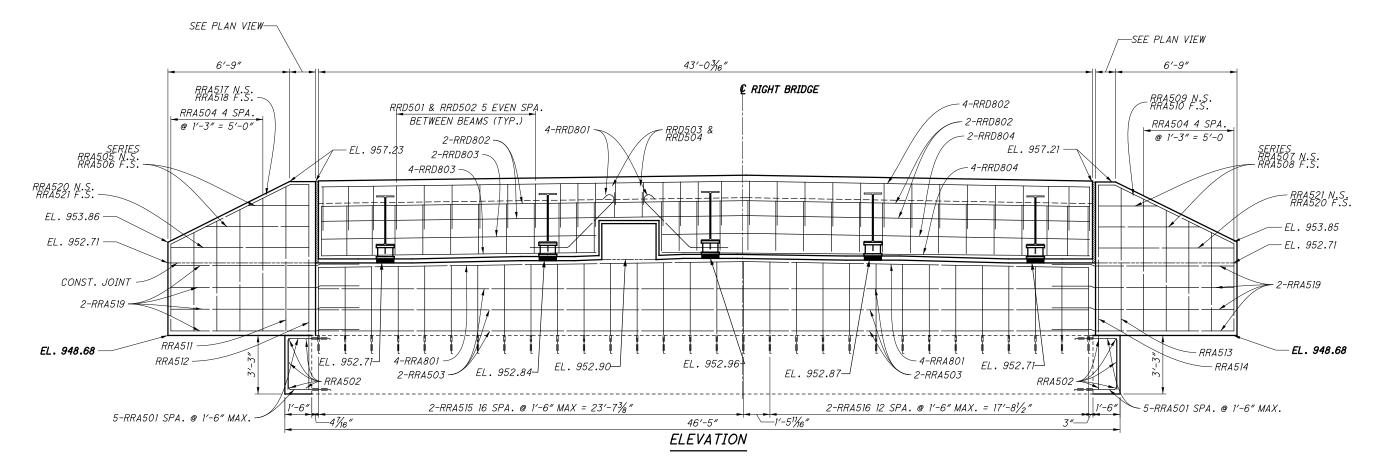
# PLAN

RIGHT BRIDGE REAR ABUTMENT

# **NOTES:**

ALL DIAPHRAGM STIRRUPS SHALL BE PLACED PARALLEL TO THE BRIDGE SKEW.

P.C.P.P. - PERFORATED CORRUGATED PLASTIC PIPE N.P.C.P.P. - NON PERFORATED CORRUGATED PLASTIC PIPE



RIGHT BRIDGE REAR ABUTMENT

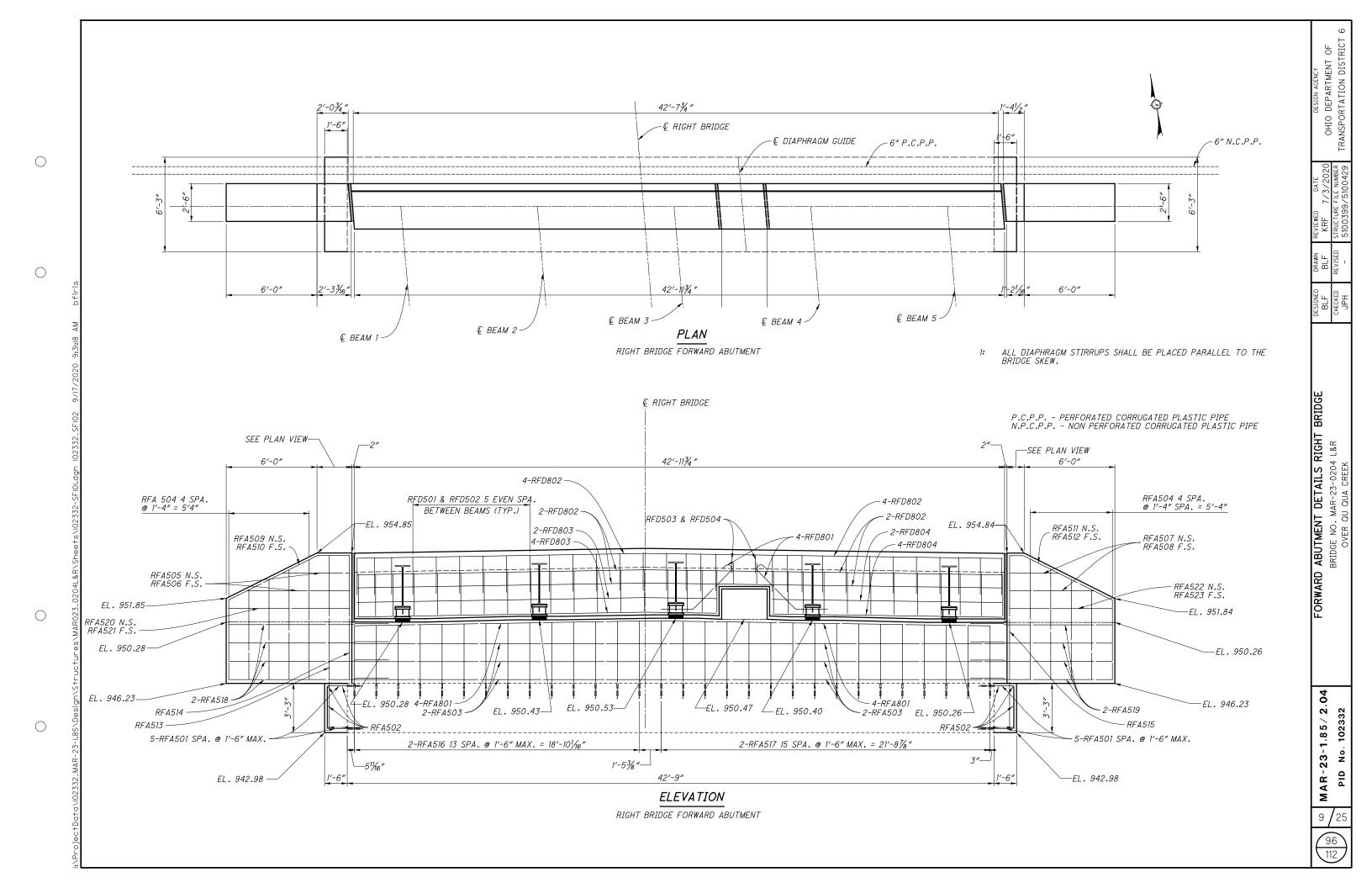
 $\bigcirc$ 

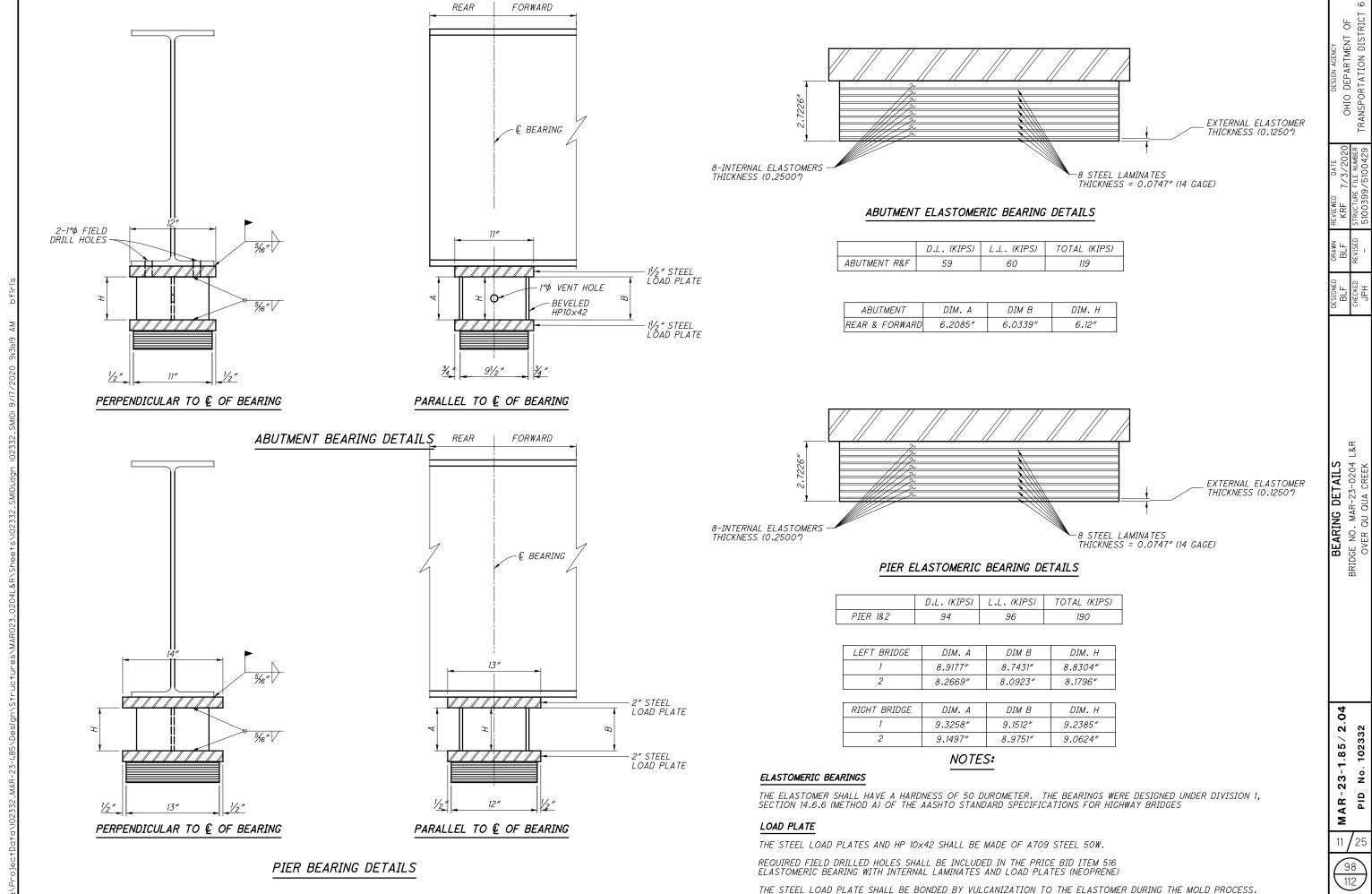
 $\bigcirc$ 

MAR-23-1.85/2.04 PID No. 102332

REAR ABUTMENT DETAILS RIGHT BRIDGE
BRIDGE NO. MAR-23-0204 L&R
OVER QU QUA CREEK

OHIO DEPARTMENT OF TRANSPORTATION DISTRICT





 $\bigcirc$ 

OHIO DEPARTMENT TRANSPORTATION DIST

Г		İ	ī	1																		_
		NUMBER			ш			DI	MENSIC	NS								IAPHR.				_
	MARK		LENGTH	WEIGHT	TYPE			<i>D</i> 11	TENSIC	7113			D801	35	4'-9"	444	18	2'-7"	1'-0"	1'-0"	<b>—</b>	╄
		TOTAL				_		С	_	-		TNC									<del></del>	╄
L						A	В	٠	D	E	R	INC	LRD501	36	11'-5"	429	3	2'-8"	2'-9"		<del></del>	╄
				IFFT	DEAD	ABUTM	- FNT	•	•				LRD502	36	6'-9"	253	2	2'-2"	2'-5"		⊢—	╀
⊢	1.04504	1 20	41.4411			2'-3"				1			LRD503	2	7'-5"	15	3	2'-8"	0'-9"		<del></del>	+
H	LRA501 LRA502	20 10	4'-11" 5'-9"	103 60	STR	2-3	2'-10"						LRD504	2	5'-7"	12	2	2'-2"	1'-10"		<del></del>	+
	LRA502 LRA503	12	27'-9"	347	STR								LRD801	8	6'-8"	142	18	3'-8"	1'-7"	1'-6"		+
F	LNAJUJ	1	14'-10.7"	347	31K		4'-11"						LRD802	16	29'-1"	1,242	STR	3-0	'''	1-0	<u> </u>	╁
	LRA504	SERIES OF	TO	92	3	2'-2"	TO					1'-4.3"	LRD803	6	24'-1"	386	STR					+
F		5	20'-4"				7'-8"					·	LRD804	6	24'-4"	390	STR					t
		1	14'-10"				4'-11"			1				-								T
	LRA505	SERIES OF	ТО	90	3	2'-2"	ТО					1'-3"	<u> </u>	l	SUBTOTAL	3,313						T
		5	19'-10"				7'-5"								Į.	LEFT FOR	DW/ADI	DTAD	HDAGM			_
		1	3'-6"										5004		4'-9"					1'-0"		_
Г	LRA506	SERIES OF	TO	10	STR							2'-4"	D801	35	4-9	444	18	2'-7"	1'-0"	7-0	<del></del>	⊬
		2	5'-10"										LFD501	34	11'-5"	405	3	2'-8"	2'-9"		<del></del>	╁
		1	3'-4"										LFD501	34	6'-9"	239	2	2'-2"	2'-5"		<del></del>	╁
L	LRA507	SERIES OF	то	9	STR							2'-4"	LFD502	2	7'-5"	15	3	2'-8"	0'-9"		$\vdash$	+
L		2	5'-8"										LFD504	2	5'-7"	12	2	2'-2"	1'-10"			+
	LRA508	1	9'-0"	9	19	1'-8"	6'-6"	3'-4"					27 5004	-	,				' ' <b>`</b>			+
⊢	LRA509	1	8'-4"	9	19	1'-6"	6'-6"	3'-4"					LFD801	8	6'-8"	142	18	3'-8"	1'-7"	1'-6"		t
- ⊦		1	3'-11"									21.411	LFD802	16	29'-1"	1,242	STR					t
F	LRA510	SERIES OF	TO	11	STR							2'-4"	LFD803	6	24'-3"	388	STR					t
⊢		2	6'-3"							<b>.</b>		-	LFD804	6	24'-5"	391	STR					T
H	LRA511	1 SERIES OF	4'-1" TO	11	STR							2'-4"										Г
⊢	LKASII	2	6'-5"	<del>  ''</del>	SIK							2-4			SUBTOTAL	3,278						Г
- H	LRA512	1	8'-3"	9	19	1'-10"	5'-9"	2'-11"								LFF	T BRID	GE DEC	CK			
<b> </b>	LRA513	1	8'-6"	9	19	2'-1"	5'-9"	2'-11"		<del>                                     </del>			LS401	542	9'-0"	3,259	16	8'-6"				_
⊢	LRA513	1	27'-10"	29	3	2'-2"	11'-5"						LS401 LS402	139	3'-9"	3,259	16	0'-7"	3'-3"			+
⊢	LRA514	2	11'-5"	24	1	2'-2"	9'-5"	<b> </b>	<del>                                     </del>	<del>                                     </del>	<u> </u>	$\vdash$	LS402 LS403	139	3'-11"	364	19	3'-3"	0'-6"	0'-6"		+
H	LRA516	1	27'-2"	28	3	2'-2"	11'-1"						LS404	139	3'-6"	325	1	0'-7"	3'-0"	J-V		+
	LRA517	1	20'-8"	22	3	2'-2"	7'-10"						LS405	139	3'-8"	340	19	3'-0"	0'-6"	0'-6"		$\vdash$
		2	7'-1"				4'-10"						LS406	402	40'-0"	10,741	STR					t
	LRA518	SERIES OF	то	198	1	2'-8"	то					0'-0.4"	LS407	58	18'-5"	714	STR					t
		13	7'-6"				5'-0"															$\vdash$
		2	7'-1"				4'-7"						LS501	552	27'-11"	16,073	16	27'-6"				T
	LRA519	SERIES OF	ТО	335	1	2'-8"	TO					0'-0.2"	LS502	552	28'-0"	16,121	STR					T
		22	7'-6"				5'-2"						LS503	213	40'-0"	8,886	STR					Г
	LRA520	8	10'-8"	89	STR								LS504	71	18'-0"	1,333	STR					Г
L	LRA521	8	10'-5"	87	STR																<u></u>	
L	LRA522	1	8'-1"	8	STR										SUBTOTAL	58,504					L	
L	LRA523	1	7'-10"	8	STR											LEFT B	RIDGE	PARA	PETS			
	LRA524	1	7'-7"	8	STR								LP501	364	7'-4"	2,784	23	0'-11"	3'-3"	3'-0"	$\overline{}$	$\top$
	LRA525	1	7'-10"	8	STR								LP502	100	4'-6"	469	STR	· ,,				H
- ⊦	1.004		001.00	200	070					<b>.</b>			LP503	24	40'-0"	1,001	STR					t
⊢	LRA801	8	29'-2"	623	STR								LP504	4	6'-5"	27	STR					T
⊢			SUBTOTAL	2,236									LP505	8	21'-9"	181	STR					T
			SUBTUTAL		DIAZAD	D 4 D//	FAFFAIT	<u> </u>					LP506	8	10'-8"	89	STR					
L				LEFT FO	KWAK								LP507	16	21'-2"	353	STR					
	LFA501	20	4'-11"	103	1	2'-3"	2'-10"						LP508	16	10'-0"	167	STR				L	
- ⊦	LFA502	10	5'-9"	60	STR								LP509	16	5'-8"	95	25	1'-10"	2'-5"	1'-5"	0'-1.5"	┺
F	LFA503	12	27'-7"	345	STR		=1.011						LP510	16	5'-8"	95	STR				<b>——</b>	┺
- ⊦	154504	1	14'-11"		<b>—</b>	0/ 0//	5'-0"					41.4.50									——	╄
⊢	LFA504	SERIES OF	TO 20'-5"	92	3	2'-2"	TO 7'-9"					1'-4.5"	LP601	50	4'-6"	338		41.00			<del></del>	╄
⊢		5	20-5 14'-11"				5'-0"						LP602	364	2'-5"	1,321	1	1'-0"	1'-6"	01.4411	⊢—	╀
H	LFA505	SERIES OF	TO	93	3	2'-2"	TO					1'-5.5"	LP603	364	3'-2"	1,731	28	1'-6"	1'-0"	0'-11"	<del></del>	⊢
⊢	L. A000	5	20'-9"	"	ا ا	1	7'-11"					, -5.5	LP604 LP605	2 2	6'-6" 10'-6"	20 32	STR STR		<b>-</b>		<del></del>	+
⊢	LFA506	1	6'-0"	6	STR		<del>, -,,</del>		<b>†</b>				LP605 LP606	2	10'-5"	32	STR		$\vdash$			+
	LFA507	1	5'-10"	6	STR			l					LP607	32	4'-0"	192	1 1	1'-0"	3'-1"			+
F	LFA508	1	8'-7"	9	19	2'-6"	5'-10"	1'-10"					2, 30,	8	3'-11"	102	<u> </u>		3'-0"			+
F	LFA509	1	8'-4"	9	19	2'-3"	5'-10"	1'-10"					LP608	SERIES OF	то	631	1	1'-0"	то		ı	1
F		1	4'-1"											12	4'-10"				3'-11"		ı	1
	LFA510	SERIES OF	то	12	STR							3'-1"							i -			T
		2	7'-2"											•	SUBTOTAL	9,557			i			Т
		1	4'-4"																i			$\Box$
	LFA511	SERIES OF	то	12	STR							3'-2"							l			
		2	7'-6"												SUBSTRUCTURE	4,475						Γ
L	LFA512	1	8'-0"	8	19	1'-1"	6'-2"	3'-1"						S	UPERSTRUCTURE	74,652						Г
L	LFA513	1	8'-2"	9	19	1'-3"	6'-2"	3'-1"	ļ	-	ļ				GRAND TOTAL	79,127						$\Gamma$
⊢	LFA514	1	27'-2"	28	3	2'-2"	11'-1"	<b>_</b>											L			$\Gamma$
⊢	LFA515	2	11'-1"	23	1	2'-2"	9'-1"	<b> </b>	-	-				<del></del>								
⊢	LFA516	1	27'-8"	29	3	2'-2"	11'-4"	-	-	-												
	1 EA547	2 SERIES OF	7'-1"	227	-	27.07	4'-6"	-	-			0,00"										
	LFA517	SERIES OF	TO 7' 7"	337	1	2'-8"	TO 5'-0"	-	-	1		0'-0.3"										
-		22	7'-7" 7'-4"	<b> </b>	-		5'-0" 4'-9"		-	-												
⊢	LFA518	SERIES OF	7'-4" TO	217	1	2'-8"	4'-9" TO					0'-0.2"										
⊢	LFA018	SERIES OF	7'-6"	21/	1	2-8"	4'-11"			-		0 -0.2"										
⊢	LFA519	8	10'-8"	89	STR	-	4-11"	-	-	-	-	<del>                                     </del>										
⊢	LFA519 LFA520	8	10'-8"	89 84	STR	-	<b>-</b>	-	1	1	1											
⊢	LFA520 LFA521	2	8'-0"	17	STR		<del> </del>	<b> </b>	<del> </del>	t		<del>                                     </del>										
⊢	LFA521	2	7'-10"	16	STR					1												
⊢	LFA523	1	7'-70	8	STR			l														
H	LFA524	1	7'-5"	8	STR		<b> </b>	1	1													
		<u>'</u>	1	l -	<u></u>			<b> </b>														
H	LFA801	8	29'-0"	619	STR		1	1	1		1											
		†	† <del></del>	†	† <u></u>		1	<b>l</b>		1												
 			SUBTOTAL	2,239			1	1	1		1											
				· -,-·-	•	•	•	•	•	•	•	•										_

I FFT	REAR D	TAPHR	AGM						ဖ
		2'-7"		41.0"	1				
444	18	2-1"	1'-0"	1'-0"					OF RIC
429	3	2'-8"	2'-9"	1	1		<del>                                     </del>		
429 253	2	2'-8"	2'-9"		1		<del>                                     </del>	_    .	
253 15	3	2'-8"	0'-9"	1	1		<del>                                     </del>		NG WE
12	2	2'-2"	1'-10"						DESIGN AGENCY OHIO DEPARTMENT OF TRANSPORTATION DISTRICT
	1 -		T		1			<u> </u>	NS 4 ZI I
142	18	3'-8"	1'-7"	1'-6"	1				SIC PEF
,242	STR			<u> </u>				$A = \begin{bmatrix} A & B \end{bmatrix}$	
386	STR							1	일 &
390	STR				İ				H
	1							<u>TYPE-1</u>	ઁ ﴿
,313								<u> </u>	≝
	RWARI	DIAD	UDACN		-				
	_							, A _ ,	S # 6
444	18	2'-7"	1'-0"	1'-0"					100 F
405	+	0/ 0//	0/ 0//						DATE 3/2( E NUM 1004
405	3	2'-8" 2'-2"	2'-9"						
239	2		2'-5"					1	7 9/
15 12	2	2'-8"	0'-9" 1'-10"	-	<b> </b>				B 88 B
12	+ -	Z-Z	1 - 10	-			<del>                                     </del>		REVIEWED DATE KRF 7/3/2020 STRUCTURE FILE NUMBER 5100399/5100429
142	18	3'-8"	1'-7"	1'-6"	<b> </b>				[항조[[[양]
,242	STR	3-0	1-/"	1 -0					- W - 1
	STR			1	<b> </b>				<sub>~</sub>   <sub>~</sub>
388 391	STR			-				$A \rightarrow A \rightarrow$	DRAWN BLF REVISED
JJ 1	SIK	-	-	1	1		<del>                                     </del>		R & P
,278	+			-				<u>TYPE-3</u>	
		L			I		ll		
	T BRID		CK_						DESIGNED BLF CHECKED JPH
,259	16	8'-6"							
348	1	0'-7"	3'-3"						B TE
364	19	3'-3"	0'-6"	0'-6"					$\vdash \vdash \vdash$
325	1	0'-7"	3'-0"					A	j l
340	19	3'-0"	0'-6"	0'-6"					j l
0,741	STR								j l
714	STR								j l
									j l
6,073	16	27'-6"							j l
5,121	STR							<u>TYPE-16</u>	j l
,886	STR							<u> </u>	j l
,333	STR								j l
									j l
3,504									1
LEFT	BRIDGE	PARA	PETS	· · · · · · · · · · · · · · · · · · ·				$A \rightarrow B$	1
,784	23	0'-11"	3'-3"	3'-0"				<u> </u>	ا ي ديا
469	STR	T			İ			\  \  \  \  \  \  \  \  \  \  \  \	l‰ ∐
,001	STR				1			\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	
27	STR				Ì				اشٰ ۾ با
181	STR								ᄩᄼᇎ
89	STR				İ			<u> </u>	
353	STR								lo ∑∄
167	STR							<u> </u>	lo ặ 이
95	25	1'-10"	2'-5"	1'-5"	0'-1.5"	0'-5"			IS ∑ ≳I
95	STR							<u>_A_</u>	ည္က ႏွင္စု
								   <del> </del>	ᄩᇎᆔ
338									<u> ビ</u> 병이
,321	1	1'-0"	1'-6"					<u>TYPE-23</u>	<b> ≧</b> ĕ
,731	28	1'-6"	1'-0"	0'-11"				<u> </u>	REINFORCING STEEL LIST BRIDGE NO. MAR-23-0204 L&R OVER QU QUA CREEK
20	STR								-
32	STR								1
31	STR								1
192	1	1'-0"	3'-1"						j l
			3'-0"						1
631	1	1'-0"	то				0'-1"		1
	1		3'-11"				<b></b>		
	1				ļ			A	1
,557	4				ļ			<b>1</b>	1
								<u>TYPE-25</u>	
	1				ļ			111 52	1
,475	1			l					j l
1,652	1				ļ				
9,127	1				1			<u> </u>	4
				L	ļ				0
								* B	0 0 1
									🥄 ၕ
								<u> </u>	22 22
								<u> </u>	ᅟ噿 우
									1 T 🙏 1
									ကြ မို
									MAR-23-1.85/2.04 PID No. 102332
								\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	ا ي يا ا
									<del> </del>
									<del>}</del>
									24/25
								$B \rightarrow A$	24/25
								<u>TYPE-37</u>	<b> </b>
								<u> </u>	[ <del>  112    </del>
									$oldsymbol{L} oldsymbol{L} oldsymbol{L} oldsymbol{L} oldsymbol{L}$

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

MARK	NUMBER	LENGTH	WEIGHT	TYPE			DII	MENSIO	DIMENSIONS										
	TOTAL			<u> </u>	A	В	С	D	E	R	INC								
			RIGHT	REAR	ABUTN	1ENT													
RRA501	20	4'-11"	103	1	2'-3"	2'-10"													
RRA502	10	5'-9"	60	STR		- ''													
RRA503	12	22'-10"	286	STR															
	2	14'-8"				4'-10"													
RRA504	SERIES OF	то	180	3	2'-2"	то					1'-3.5"								
MAJOT	5 5	19'-10"	700	ľ	***	7'-5"					1 -3.3								
	1	3'-6"				<b>⊢</b> ′~													
RRA505	SERIES OF	то	10	STR							2'-4"								
KKASUS		5'-10"	10	311							2-4								
	1	3'-4"																	
554500			•								0.411								
RRA506	SERIES OF	то	9	STR							2'-4"								
	2	5'-8"																	
	1	3′-4″	_																
RRA507	SERIES OF	то	9	STR							2'-4"								
	2	5′-8″																	
	1	3'-6"																	
RRA508	SERIES OF	то	10	STR							2'-5"								
	2	5'-11"																	
RRA509	1	8'-5"	9	19	1'-0"	6'-8"	3'-4"												
RRA510	1	8'-8"	9	19	1'-2.5"	6'-8"	3'-4"												
RRA511	1	21'-0"	22	3	2'-2"	8'-0"													
RRA512	1	27'-8"	29	3	2'-2"	11'-4"				1									
RRA513	1	20'-10"	22	3	2'-2"	7'-11"				<b>—</b>	<del>                                     </del>								
RRA513	1	27'-8"	29	3	2'-2"	11'-4"				1									
AAA44	2		43	+ *	<del></del>					<del>                                     </del>	1								
DD454-		7'-3"	000	,	0.0"	4'-9"				1	0100								
RRA515	SERIES OF	то	263	1	2'-8"	то				1	0'-0.2"								
	17	7'-7"				5'-1"													
	2	7'-3"				4'-9"													
RRA516	SERIES OF	то	201	1	2'-8"	то					0'-0.3"								
	13	7'-7"				5'-1"													
RRA517	1	8'-8"	9	19	1'-2.7"	6'-8"	3'-4"												
RRA518	1	8'-6"	9	19	1'-0.2"	6'-8"	3'-4"												
RRA519	16	10'-4"	172	STR															
RRA520	2	7'-9"	16	STR															
RRA521	2	7'-7"	16	STR															
MADE	-	, -,	,,,	U/N															
RRA801	8	24'-3"	518	STR															
KKAOUT		24-3	310	311							1								
		SUBTOTAL	4.004	-															
		SUBTUTAL	1,991																
			RIGHT F	ORWAR	RD ABU	IMENI				_									
RFA501	20	4'-11"	103	1	2'-3"	2'-10"													
RFA502	10	5'-9"	60	STR															
RFA503	12	23'-0"	288	STR															
	2	15'-6"				5'-3.1"													
RFA504	SERIES OF	то	189	3	2'-2"	то					1'-4"								
	5	20'-10"				7'-11.1"													
	1	4'-4.5"																	
RFA505	SERIES OF	то	12	STR							2'-3.5"								
7.11 / / / / / /	2	6'-8"		""							- 0.0								
	1	4'-2.5"			<del>                                     </del>	<del>                                     </del>				<del>                                     </del>	1								
DEVEN	SERIES OF		44	070	1	1				1	2'-4"								
RFA506		TO	11	STR	1	1				1	2-4"								
	2	6'-6.5"		-	<b></b>	<b></b>													
	1	3'-3.8"	_		l	1				1									
RFA507	SERIES OF	то	9	STR	l	1				1	2'-4"								
	2	5'-7.7"																	
	1	3'-5.9"			1	1				1									
RFA508	SERIES OF	то	10	STR	l	1				1	2'-4"								
	2	5'-9.9"																	
RFA509	1	8'-8"	9	19	2'-1"	5'-10.5"	2'-11.3"												
RFA510	1	8'-6"	9	19	1'-10.9"	5'-10.5"	2'-11.3"			Ī									
RFA511	1	7'-7"	8	19	1'-0.2"	5'-10.5"	2'-11.3"												
RFA512	1	7'-9"	8	19	1'-2.3"	5'-10.5"	2'-11.3"												
RFA513	1	27'-11"	29	3	2'-2"	11'-5.4"													
RFA514	1	21'-4"	22	3	2'-2"	8'-2"				l									
RFA515	1	27'-10"	29	3	2'-2"	11'-5"				1									
W AOIO	2	7'-4"	23	<del>                                     </del>	<del> </del>	4'-9.7"				<del>                                     </del>	1								
DEAF40			240	_	2, 0,,					1	0100"								
RFA516	SERIES OF	TO	219	1	2'-8"	TO				1	0'-0.3"								
	14	7'-8"			ļ	5'-1.4"					1								
	2	7'-4"		l .	<b>.</b>	4'-9.6"				1	1								
RFA517	SERIES OF	то	235	1	2'-8"	то				1	0'-0.3"								
	15	7'-8"				5'-1.5"													
RFA518	8	10'-6"	88	STR															
RFA519	8	9'-9.4"	82	STR															
RFA520	1	7'-11"	8	STR															
RFA521	1	7'-9"	8	STR															
RFA522	1	6'-10.3"	7	STR	l	l				1									
RFA523	1	7'-0.3"	7	STR															
	,	, -0.5		1 J/K						1									
RFA801	8	24'-4"	520	STR	<b>-</b>	<b>-</b>				<del>                                     </del>	1								
OF MOUT	0	24 -4	<b>320</b>	J SIK		<b> </b>				<b>!</b>	1								
,				1															
7.1.7.1007		SUBTOTAL	1,970								-								

												<b>-</b>	9
			RIGHT	REAR	DIAPHI	RAGM							
D801	30	4'-9"	380	18	2'-7"	1'-0"	1'-0"					1 <u>                                    </u>	DESIGN AGENCY OHIO DEPARTMENT OF TRANSPORTATION DISTRICT
												1   1	⊢ ES
RRD501	30	11'-5"	357	3	2'-8"	2'-9"							
RRD502	30	6'-9"	211	2	2'-2"	2'-5"							
RRD503	2	7'-5"	15	3	2'-8"	0'-9"						1	10 A B
RRD504	2	5'-7"	12	2	2'-2"	1'-10"							DESIGN AGENCY DEPARTMEI RTATION DI
RRD801	8	6'-8"	142	18	3′-8″	1'-7"	1'-6"					$A \rightarrow B \rightarrow B$	
RRD802	16	24'-3"	1,036	STR									
RRD803	6	15'-3"	244	STR								TVDC 1 TVDC 0	
RRD804	6	23'-9"	380	STR								<u> TYPE-1</u>	1 ==
												]	
		SUBTOTAL	2,777									1	O = 0
		•	RIGHT FO	RWAR	D DTAF		1	022 H					
D801	35	4'-9"	444	18	2'-7"	1'-0"	1'-0"	1	1	ì			DATE 7/3/2020 FILE NUMBER 9/5100429
D001	33	4-3	777	10	2-/	1-0	1-0						51 = 13
RFD501	28	11'-5"	333	3	2'-8"	2'-9"			<u> </u>			+ $ $ $ $ $ $ $ $	7 3 6
RFD502	28	6'-9"	197	2	2'-2"	2'-5"						1   a   <del>1  </del> 1   <del>- 1</del>	REVIEWED KRF 7. STRUCTURE F.
RFD503	2	7'-9"	16	3	2'-8"	0'-11"						1	
RFD504	2	6'-1"	13	2	2'-2"	2'-0"							REVIEWED KRF STRUCTUR 510039
NI D304		0-1	13		2-2	2-0						1 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	E 0,
RFD801	8	6'-8"	142	18	3'-8"	1'-7"	1'-6"						z . 8
RFD802	16	24'-3"	1,036	STR	3-0	l '-'	' - <del>'</del> -					$A \longrightarrow A$	DRAWN BLF REVISED
RFD803	6	23'-9"	380	STR								<b> </b>	15 m 15
RFD804	6	15'-1"	242	STR	-				<del> </del>	<b>†</b>	<b>†</b>	TYPE-3 TYPF-12	
5004	•	13-1	474	3/A	<del> </del>	<b> </b>			<u> </u>	1	<del> </del>	<u> TYPE-3</u>	
		SUBTOTAL	2,803	-	-	<b> </b>				1		1	Barlar - International
		JUDIUIAL		IT 222	<u> </u>							1	DESIGNED BLF CHECKED JPH
					DGE DE	CK						]	
RS401	552	9'-0"	3,319	16	8'-6"							A R	
RS402	139	2'-10"	263	1	0'-7"	2'-4"						$A \longrightarrow A \longrightarrow A$	
RS403	139	3'-0"	279	19	2'-4"	0'-6"	0'-6"					<b>」</b>	
RS404	139	3'-2"	294	1	0'-7"	2′-8″							
RS405	139	3'-4"	310	19	2'-8"	0'-6"	0'-6"						
RS406	325	40'-0"	8,684	STR								<u> </u>	
												TVD5 10	'
RS501	552	23'-7"	13,578	16	23'-0"							<u> </u>	
RS502	552	23'-0"	13,242	STR									
RS503	275	40'-0"	11,473	STR									
RS504	275	20'-0"	5,737	STR									
												$A \cup B \cup \overline{A}$	
		SUBTOTAL	57,870									」	<b> </b> ⊢ ~
			RIGHT	BRIDG	E PARA	PETS						[	∾ <u>∞</u>
RP501	364	7'-4"	2,784	23	0'-11"	3'-3"	3'-0"	1	1	1	1		RCING STEEL LIST O. MAR-23-0204 L&R R QU QUA CREEK
RP502	100	4'-6"	469	STR	U-11	"	1 3 -0						SING STEEL   . MAR-23-0204 QU QUA CREEK
RP503	24	40'-0"	1,001	STR									STEEL R-23-020
RP504	4	6'-5"	27	STR								<u>TYPE-19</u>	
RP505	8	21'-9"	181	STR								1 11 F E - 13	\ <b>o</b> 5 ₹
RP506	8	21'-0"	175	STR								† <u></u>	ြေမွွှ
RP507	8	21'-2"	177	STR								1	REINFORCING BRIDGE NO. MAR.
RP508	16	10'-0"	167	STR								-   <sub>   </sub>	<b>⊘</b> ⊹ ~
RP509	16	5'-8"	95	25	1'-10"	2'-5"	1'-5"	0'-1.5"	0'-5"			-	lb ∠ ë
RP510	16	5'-8"	95	STR			T	1	_ ` ` _				<b> <u>₩</u> 866</b>
RP511	2	10'-5"	22	STR								<u>TYPE-23</u>	REINFORC BRIDGE NO.
RP512	2	10'-4"	22	STR								111 L 23	<b> ₩</b> ₩
RP513	2	10'-7"	22	STR									<del>-</del> -
RP514	2	10'-6"	22	STR	<b>†</b>	1				İ		1 / \ wi	
	<u>-</u>	"		† - · · · ·	<b> </b>	<b>l</b>				t		1	
RP601	50	4'-6"	338			1				<b>1</b>			
RP602	364	2'-5"	1,321	1	1'-0"	1'-6"						1   \\ \\ c. \rightarrow	
RP603	364	3'-2"	1,731	28	1'-6"	1'-0"	0'-11"			1			
RP604	2	6'-6"	20	STR								1   ,  \ B _	
RP605	1	10'-6"	16	STR									
RP606	1	10'-5"	16	STR								1 ' ' '	
RP607	32	4'-0"	192	1	1'-0"	3'-1"						<u>TYPE-25</u> _ / L <sub>v</sub>	
	8	3'-11"				3'-0"						111 50	
RP608	SERIES OF	то	631	1	1'-0"	то					0'-1"	$  \ \   \ \  _{\mathcal{C}}$	1
	12	4'-10"			L	3'-11"						]	<del> </del>
					<u> </u>							]	0°
		SUBTOTAL	9,524									]   B	2, %
												<u> </u>	1 7 8
													102332
		SUBSTRUCTURE	3,961									TYPE-28	-1.85,
	S	UPERSTRUCTURE	72,974									<u> </u>	+ 5
		GRAND TOTAL	76,935										No.
												<b>_</b>	1 (4
												-   \	1 ' ^
													AR- PID
													1 ~
													Σ
													05 /05
												$\begin{bmatrix} B \end{bmatrix} \begin{bmatrix} A \end{bmatrix}$	25/25
												<del>       </del>	
												TVDC 33	112
												<u>TYPE-37</u>	
												<del></del>	1\ 112 /

 $\bigcirc$ 

 $\bigcirc$ 

 $\bigcirc$ 

