

GENERAL INFORMATION

INTRODUCTION

The project consists of the addition of a portion of the S3 Lane, a portion of the N3 Lane, and relocation of 6.4 miles of USR 23, beginning 1000 feet north of the Marion-Wyandot County line, extending northwestward, generally adjacent to existing USR 23, and terminating on existing USR 23, 3000 feet south of Co. Rd. C2. Included in the report are profiles of the SR 294 Interchange and associated ramps, Co. Rd. 127, and the Little Sandusky Creek Relocation.

Proposed grades indicate the following maximum proposed cuts and fill embankments:

	CUTS (MAX.)	FILL EMBANKMENTS (MAX.)
Mainline (USR 23)	12'	19'
SR 294 Interchange	-	12'
SR 294	-	11'
Ramp A	-	10'
Ramp B	10'	10'
Ramp C	1'	2'
Ramp D	9'	2'
Co. Rd. 127	1'	4'

Little Sandusky Creek Relocation - excavation, maximum 14 feet in depth.

GEOLOGY OF THE PROJECT

The alignment traverses a gently rolling portion of the glaciated Mississippi Valley Plain, in an area where moderately deep glacial drift, overlies carbonate bedrock, of the Monroe formation.

EXPLORATION

Exploratory borings were made by means of truck-mounted mechanical soil auger and hand auger (in areas of difficult access), between March 30 and April 7, 1964.

INVESTIGATIONAL FINDINGS

Materials occurring immediately below proposed grade consist of silt clays (A-6a and A-6b) and clays (A-7-6), having moisture contents generally in the lower portion of the plastic range, and few silts.

Frost susceptible silts were encountered within three feet below proposed grade at stations 12+75, 100+50, 205+00, and 284+91.

Wet materials were encountered at mainline stations 12+75, 188+65, 205+00, and 285+00, and SR 294 station 604+25.

In the embankment foundations, soils are predominantly comprised of silts (A-4a and A-4b), silt clays (A-6a and A-6b), and clays (A-7-6), having moisture contents generally in the lower portion of the plastic range.

Wet materials were encountered at the following stations:

17+00	77+00	208+00
41+00	91+00	240+75
44+00	96+00	305+50
72+00	106+65	270+21
	205+65	

Organic material was encountered at stations 96+00 and 106+00.

The excavation for the Little Sandusky River Relocation will be in sandy silts (A-4a) and silt clays (A-6a and A-6b).

LEGEND FOR PROJECT AVERAGE RESULTS OF TESTS— 224 SAMPLES TESTED

DESCRIPTION	H. R. B. CLASS	OHIO CLASS	% AGG.	% C. SAND	% F. SAND	% SILT	% CLAY	LIQUID LIMIT	PLASTICITY INDEX	WATER CONTENT	SAMPLES TESTED
Gravel	A-1-a(0)	A-1-a	64	18	8	-	10	NP	NP	14	2
Stone fragments with sand	A-1-b(0)	A-1-b	1	69	22	-	8	NP	NP	6	1
Coarse and fine sand	-----	A-3a	0	13	60	10	17	25	2	13	4
Sandy silt	A-4(5)	A-4a	8	6	27	31	28	24	5	17	24
Silt	A-4(8)	A-4b	1	2	6	62	29	26	4	23	31
Silt and clay	A-6(9)	A-6a	6	5	10	38	41	31	12	20	78
Silty clay	A-6(11)	A-6b	2	2	7	40	49	38	17	24	30
Clay	A-7-6(14)	A-7-6	1	1	4	36	58	45	23	27	54

Sod and/or Topsoil =X'= Approximate depth.

Berm material

Auger boring - plan view.

Auger boring plotted to vertical scale only.

Water content nearly equal to or greater than liquid limit.

Indicates a non-plastic material with a high water content.

Free water.

Static water level.

NOTE: Figures beside borings indicate water content in percent. e.g. /5

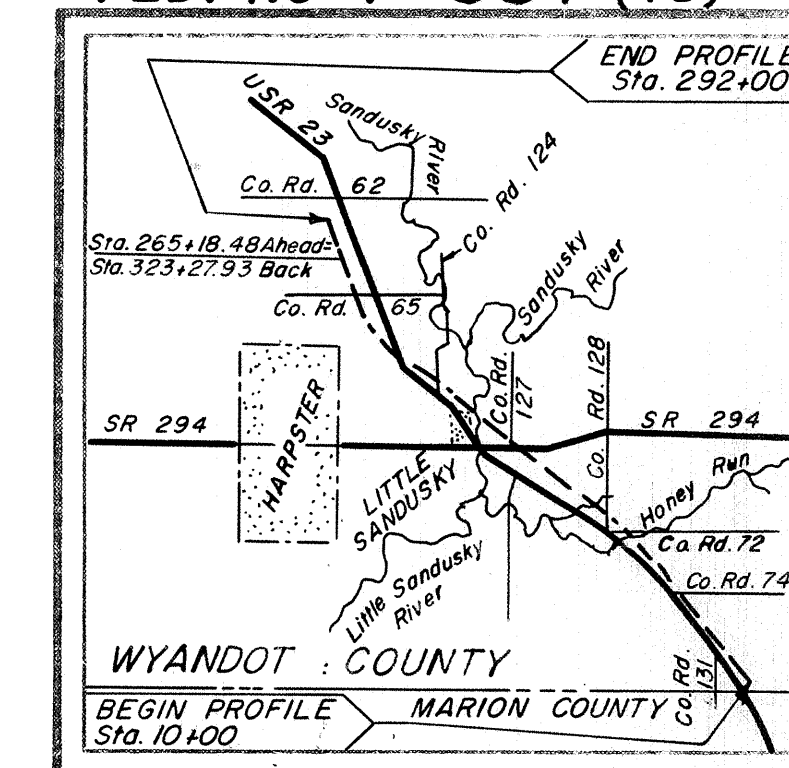
SOIL PROFILE
WYANDOT COUNTY
WYA-23-0.20

1
29

OHIO STATE HIGHWAY TESTING LABORATORY
1620 W. BROAD ST. COLUMBUS 23, OHIO

NOTE: INFORMATION SHOWN BY THIS SUBGRADE PROFILE WAS OBTAINED SOLELY FOR USE IN ESTABLISHING DESIGN CONTROLS FOR THE PROJECT. THE STATE OF OHIO DOES NOT GUARANTEE THE ACCURACY OF THIS DATA AND IT IS NOT TO BE CONSTRUED AS A PART OF THE PLANS GOVERNING CONSTRUCTION OF THE PROJECT.

FED. NO F-607 (10)



LOCATION MAP

Recon - J.S.M. 3/20/64
Drilling - A.P., B.D.L. 3/30/64 to 4/7/64
Drafting - A.F., S.P.W. 7/9/64

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SUMMARY OF SOIL TEST DATA

NOTE: NP shown in Liquid Limit and Plasticity Index columns indicates that the material is non-plastic.
*Denotes sample taken at or near grade.

Table with columns for Station & Offset, Depth, Agg., C.S., F.S., Silt, Clay, L.L., P.I., W.C., SHTL Class. The table contains multiple columns of data for various soil samples, including station numbers like 12+75, 17+00, 21+00, etc., and their corresponding soil test results.

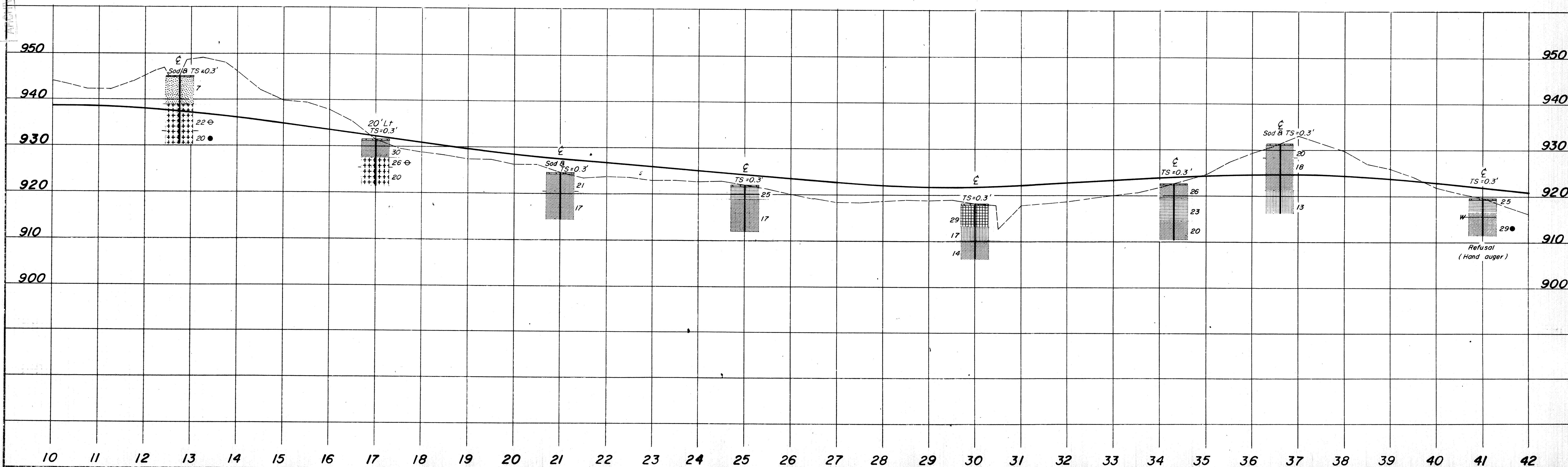
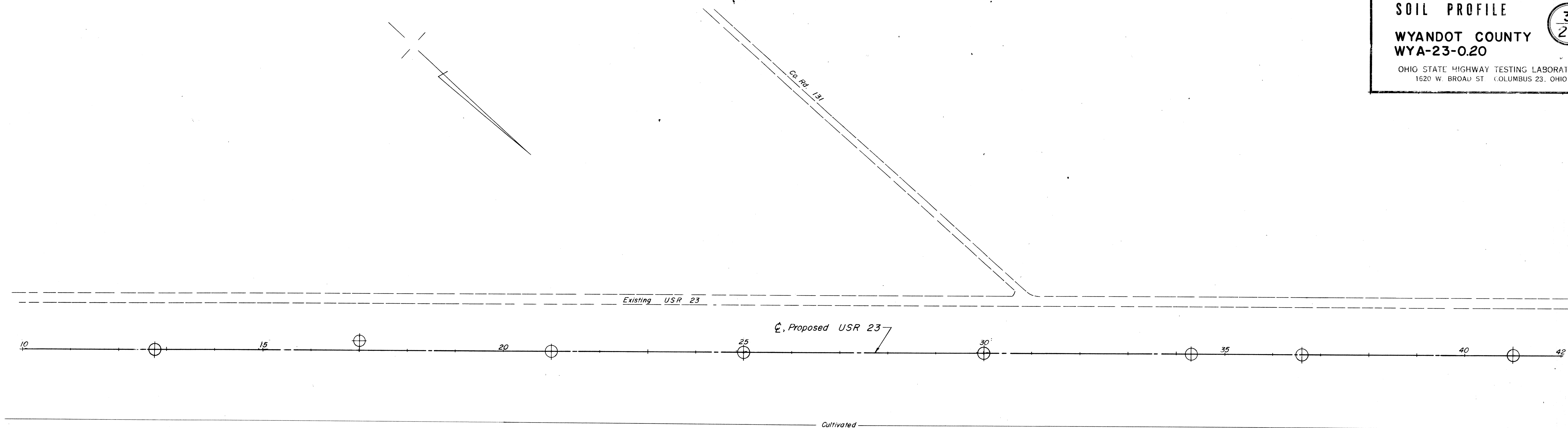
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SOIL PROFILE

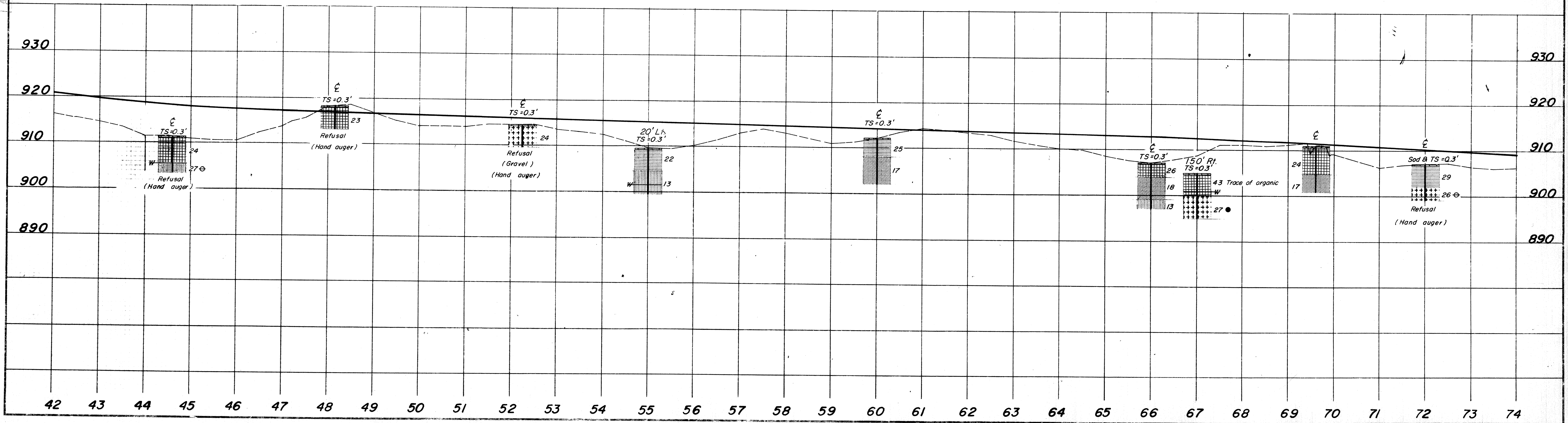
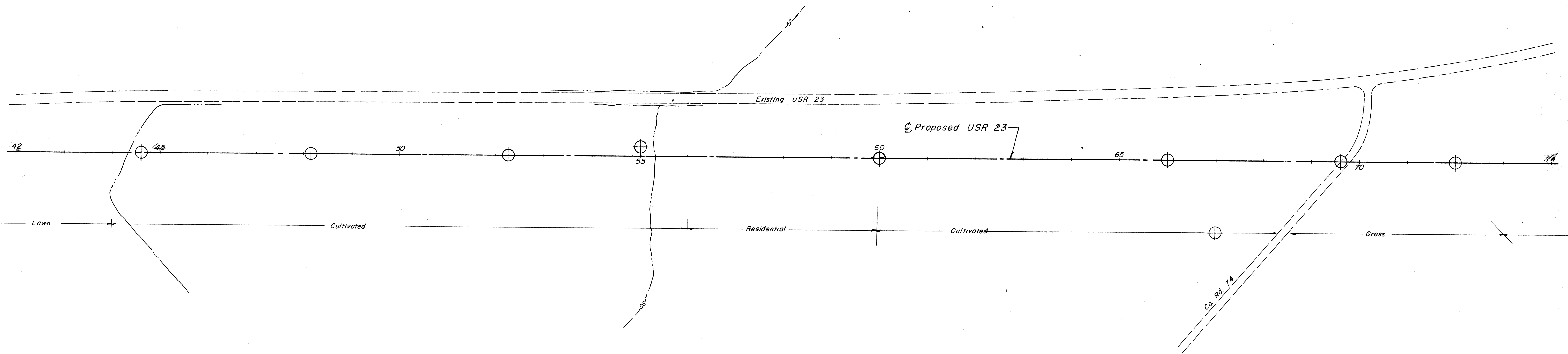
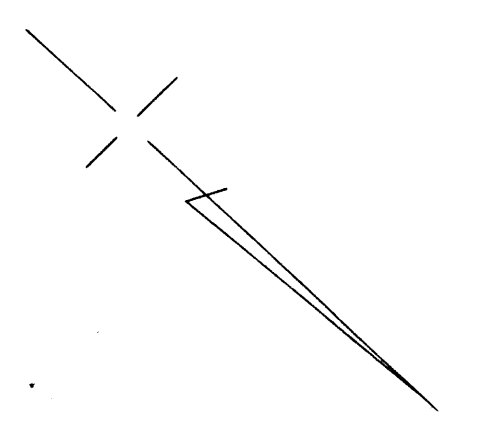
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WYA-23-0.20

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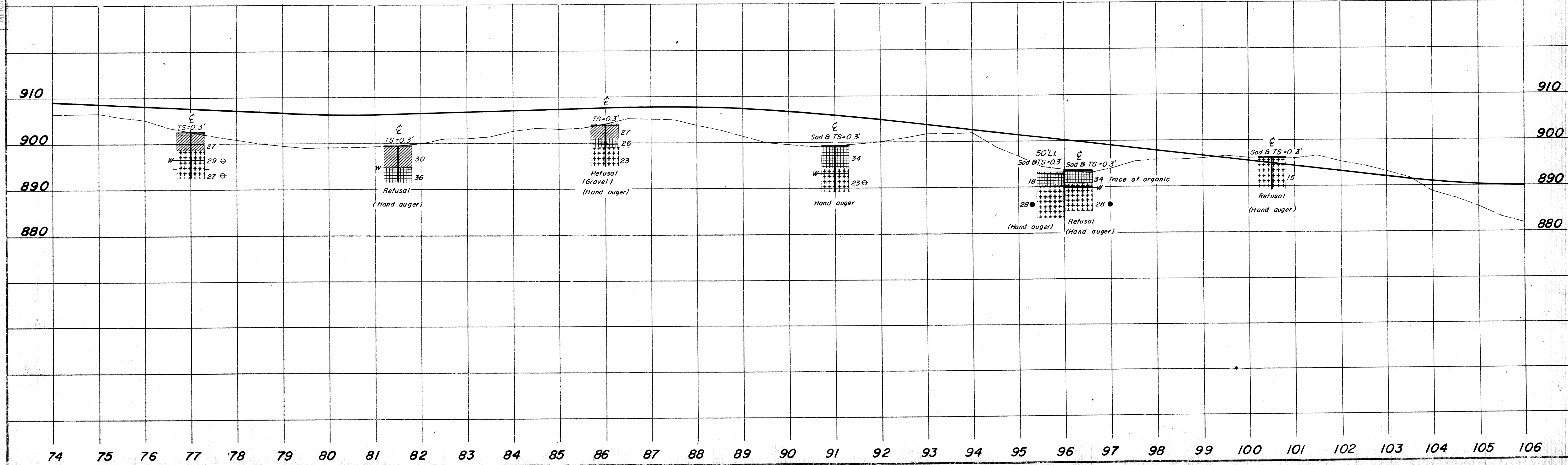
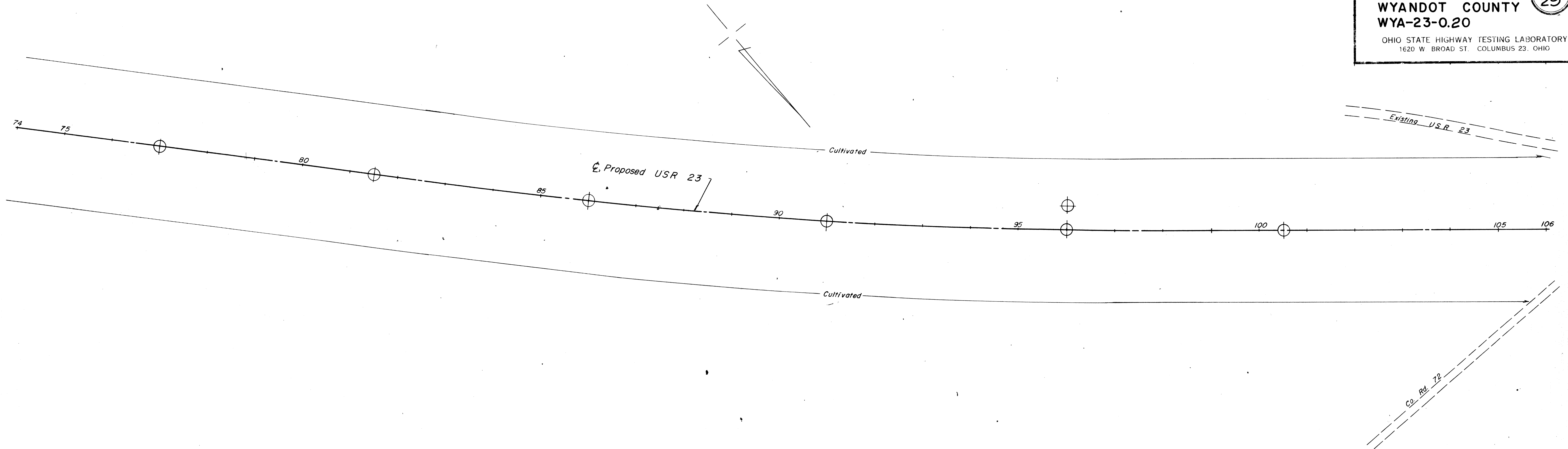
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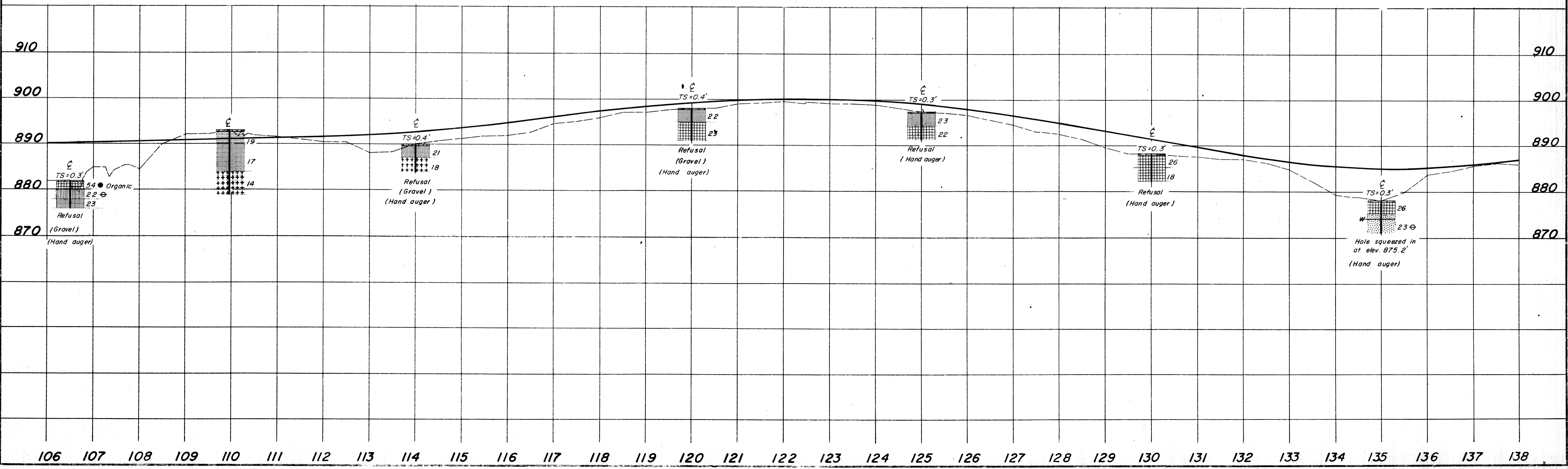
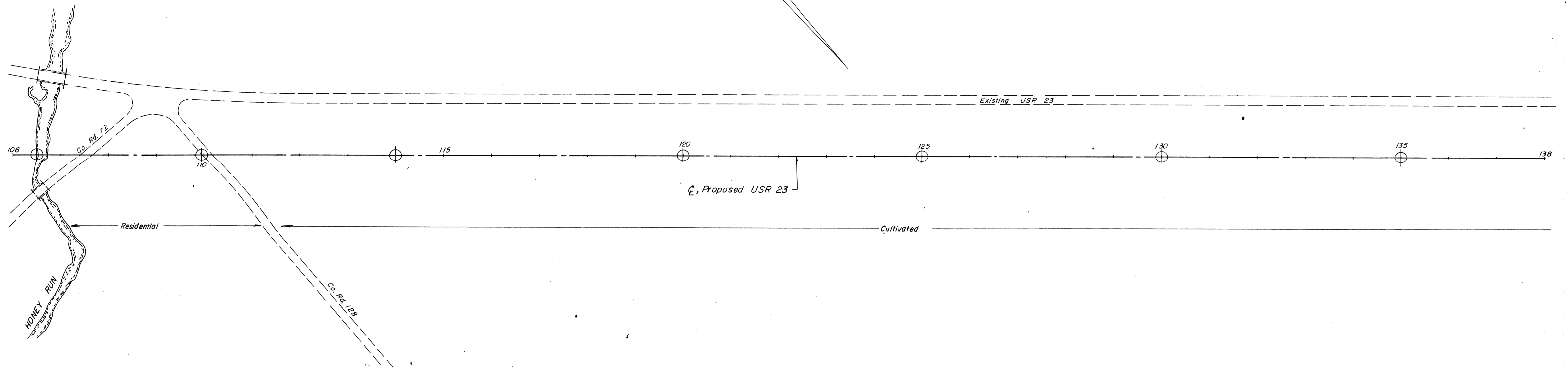
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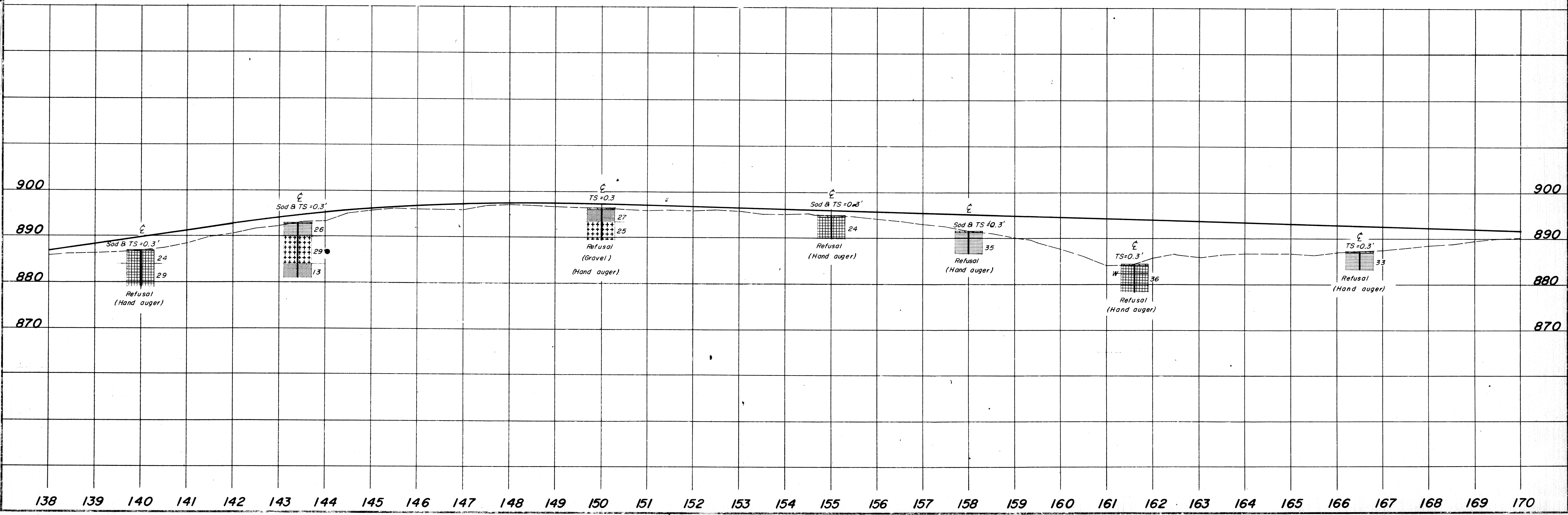
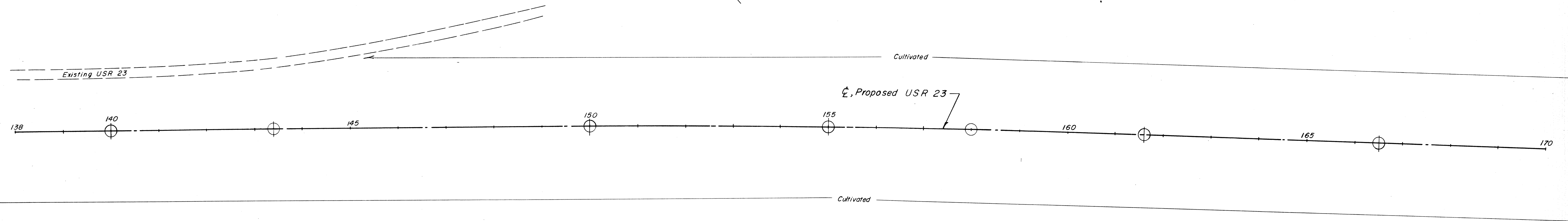
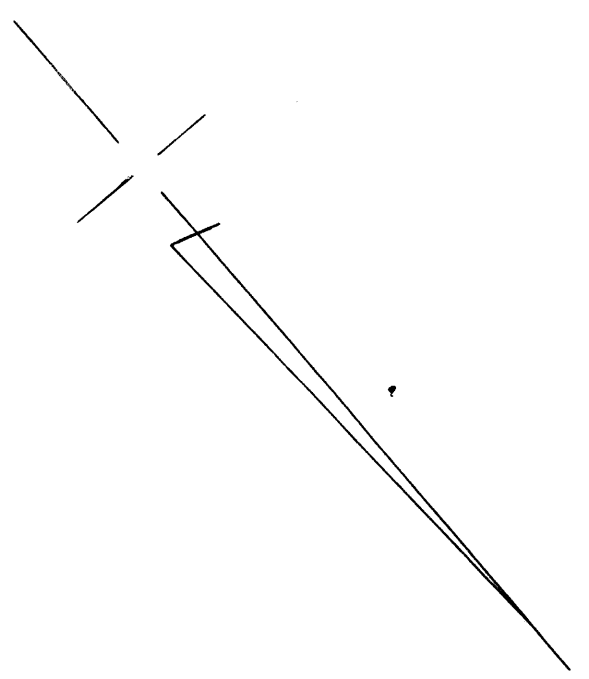
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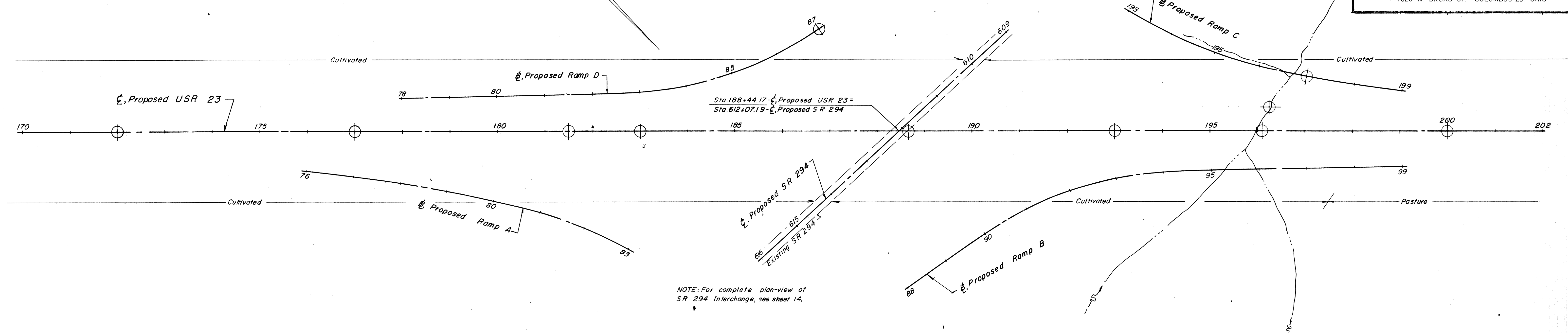
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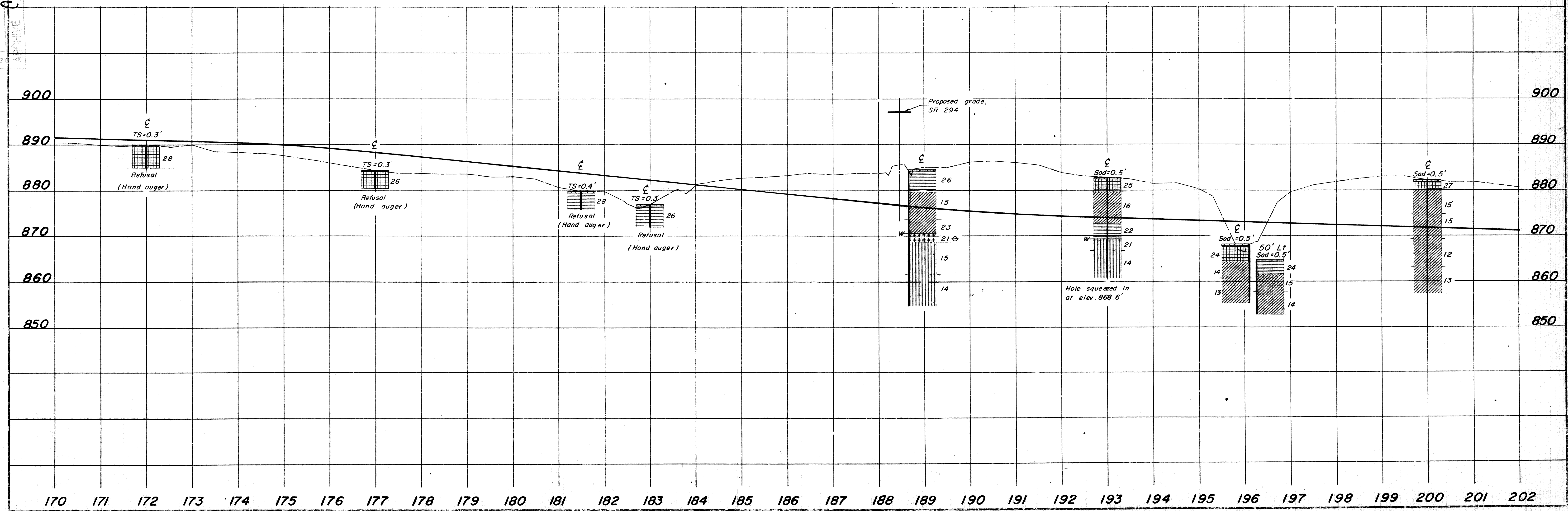


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 DISTRICT 1
 ARCHIVE 08997

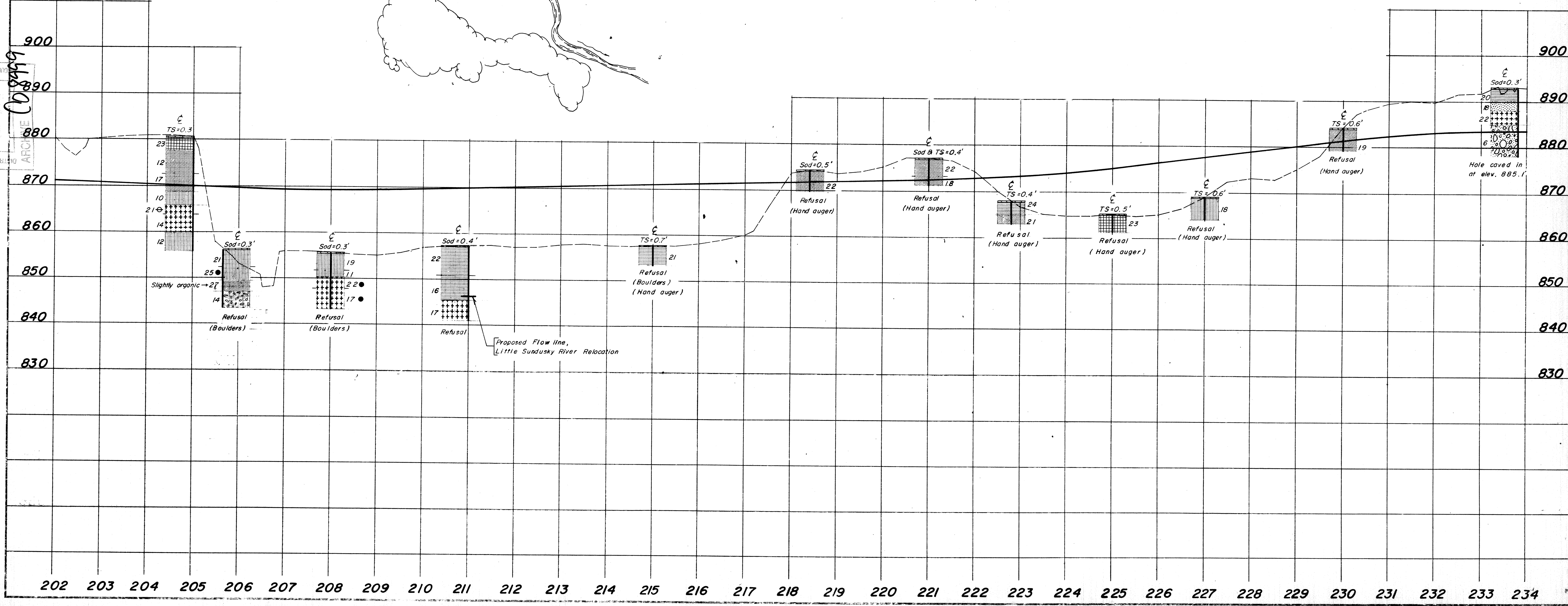
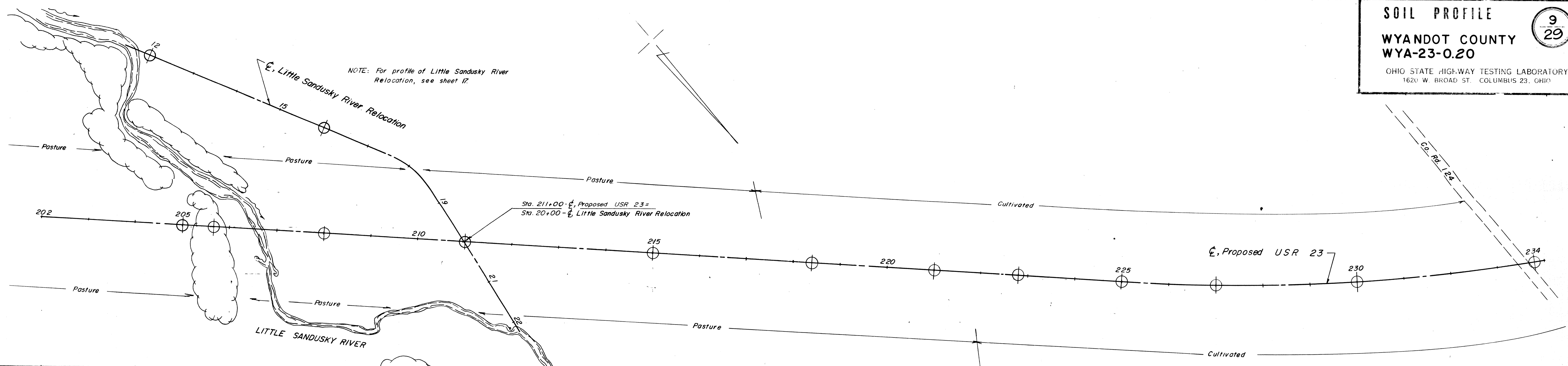


NOTE: For complete plan-view of SR 294 Interchange, see sheet 14.

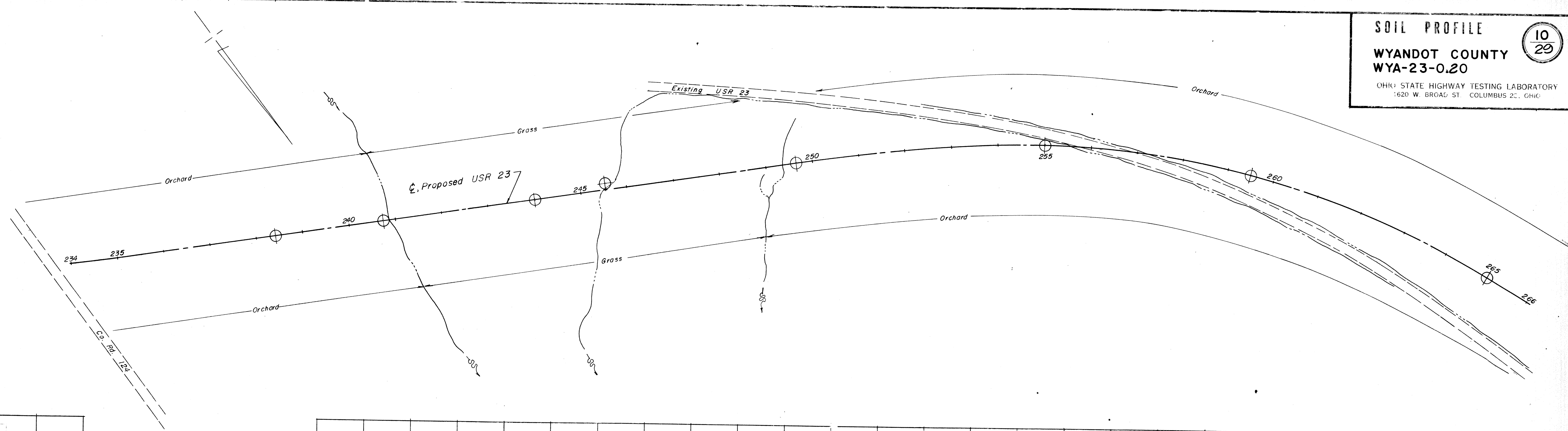
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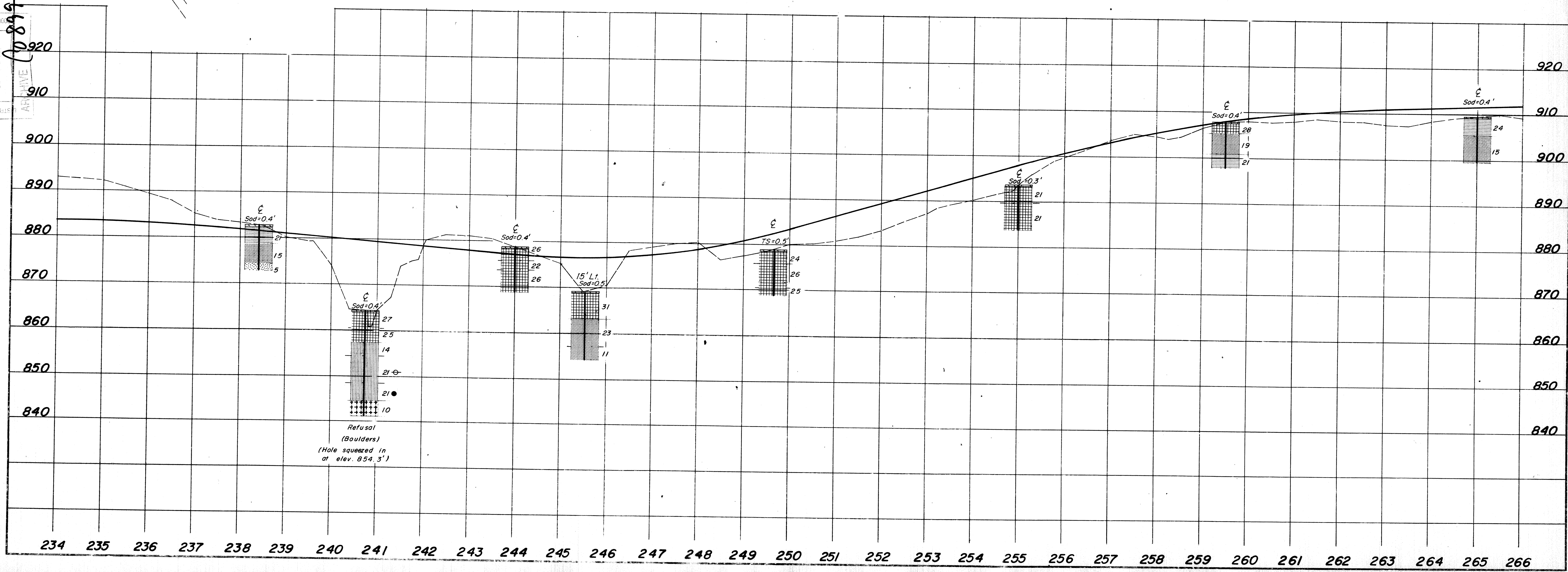
NOTE: For profile of Little Sandusky River Relocation, see sheet 17.

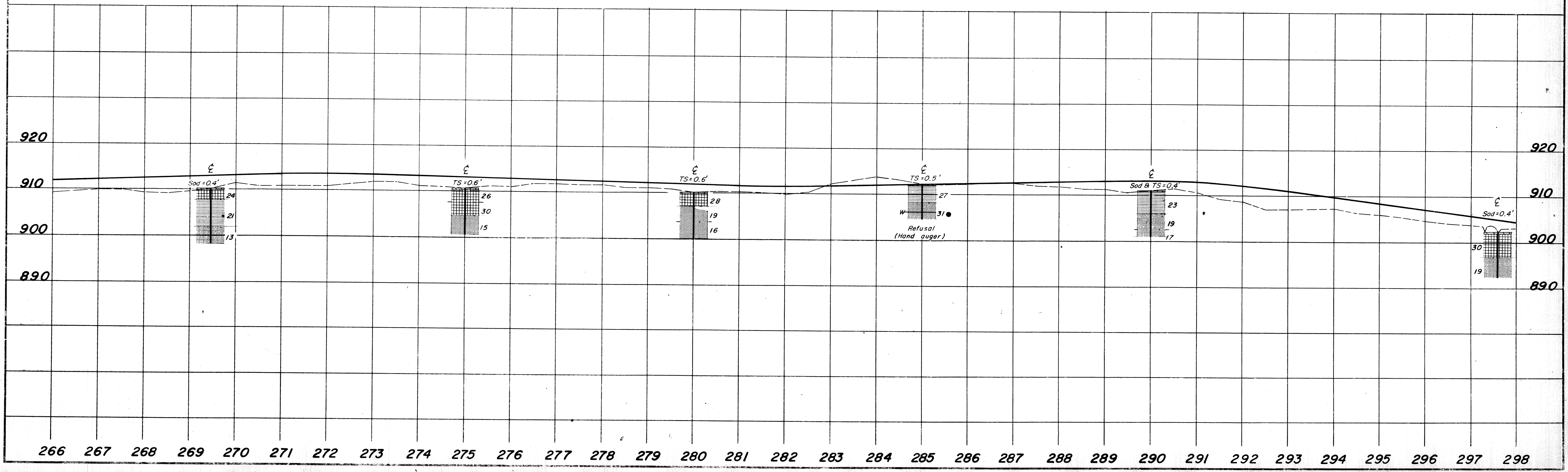
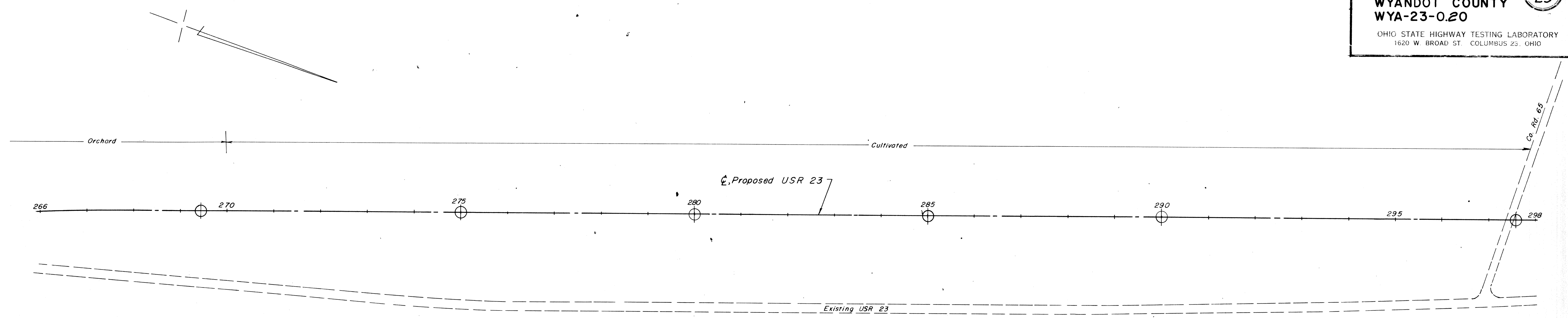


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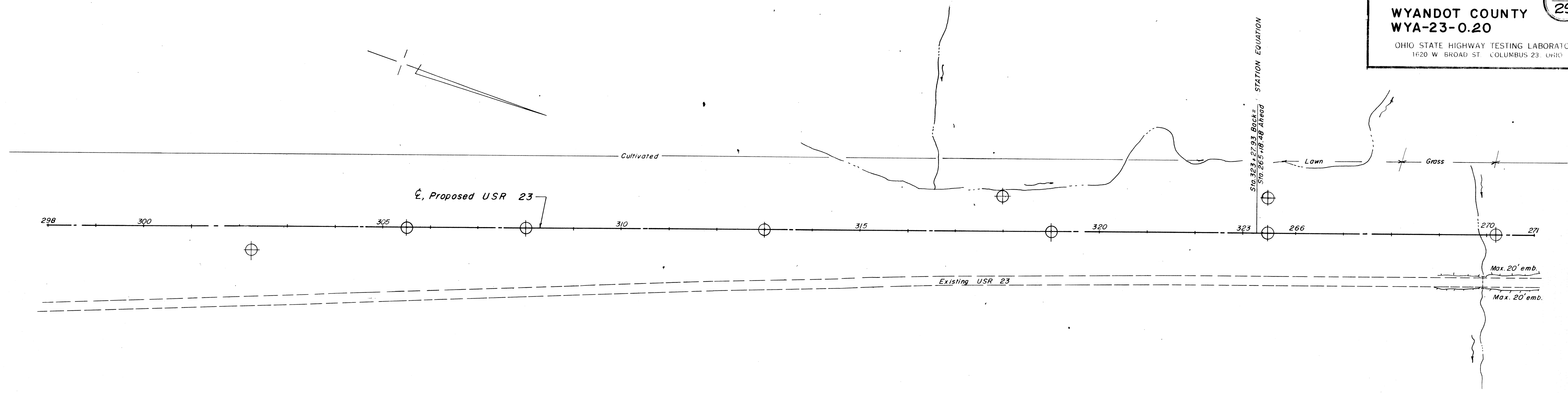


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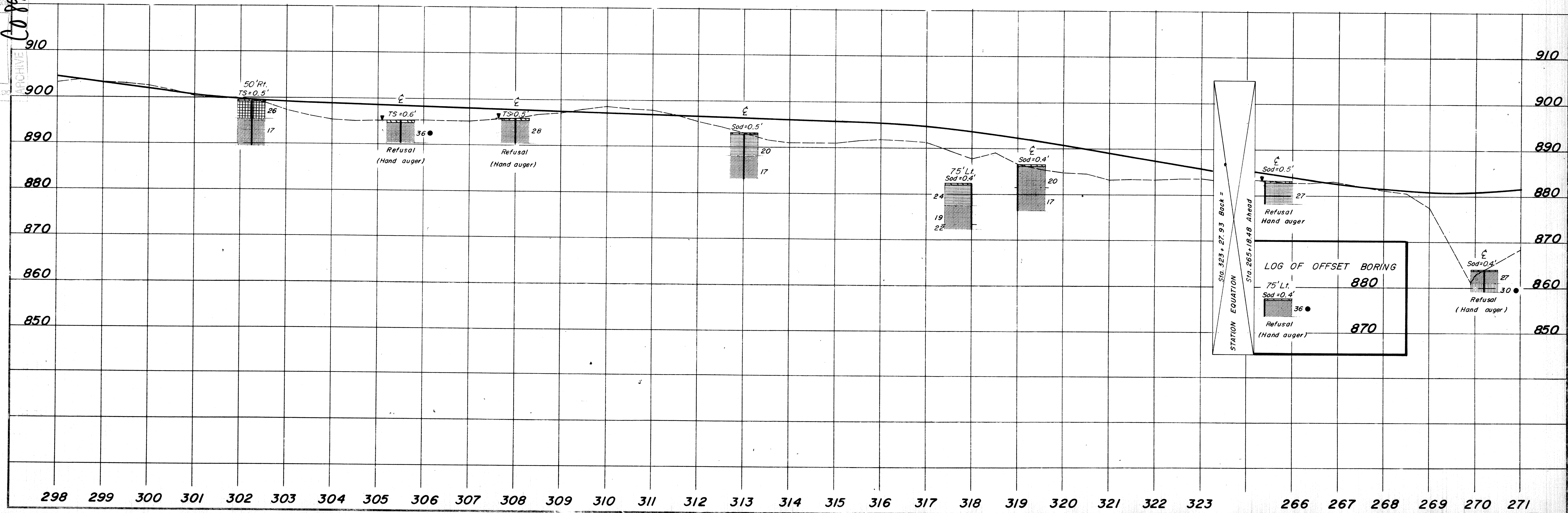




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LOG OF OFFSET BORING

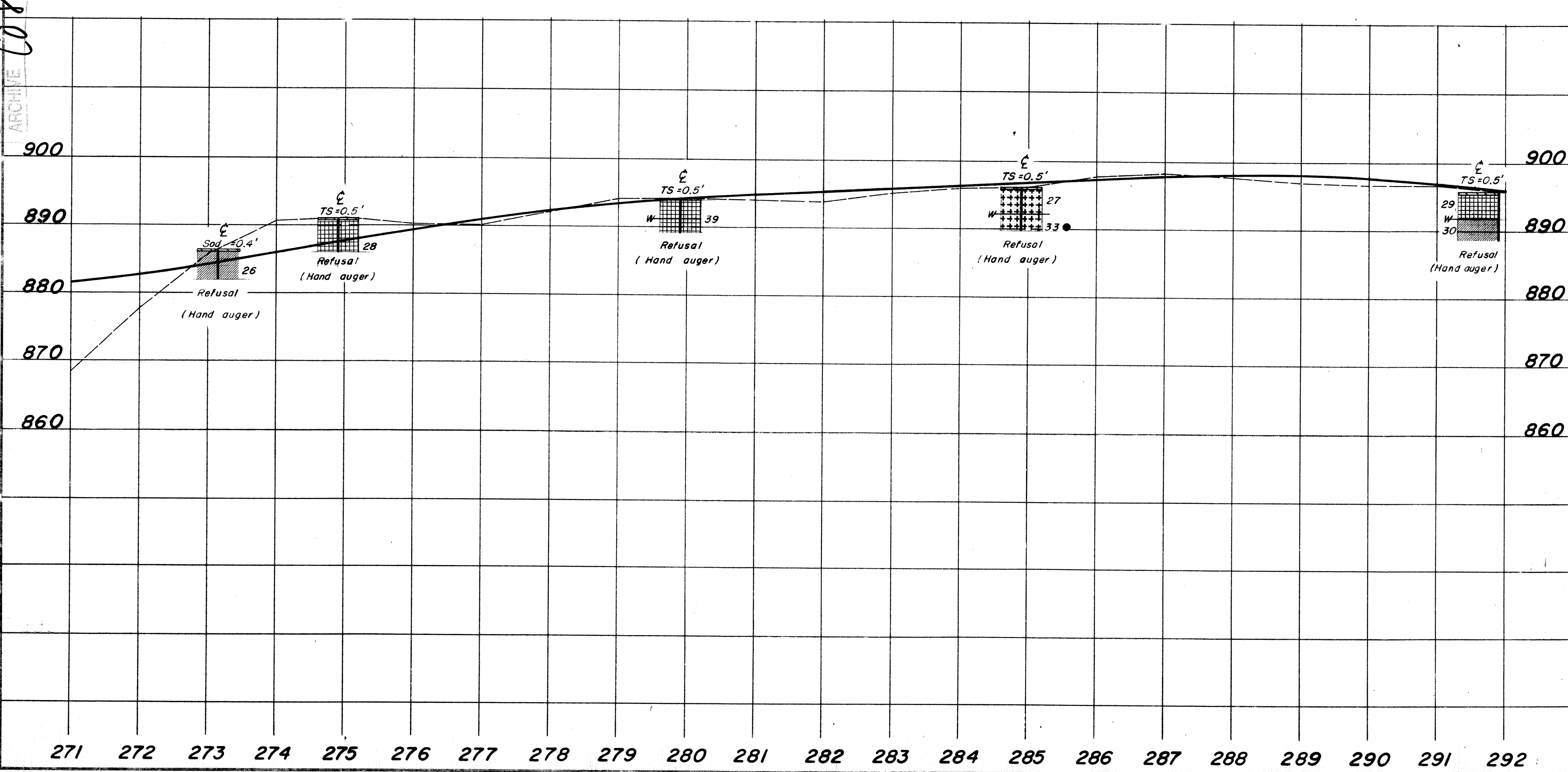
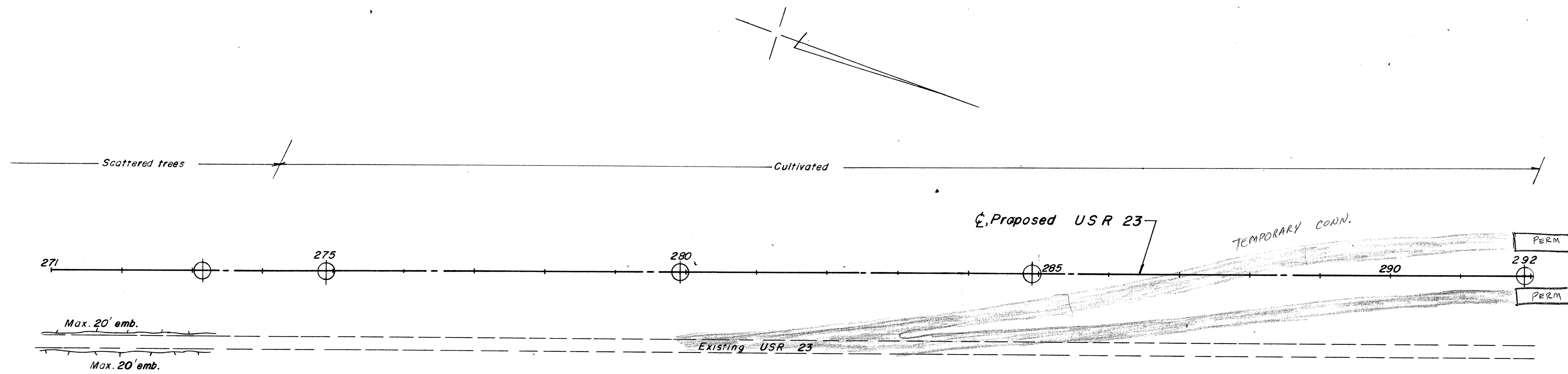
75' Lt. Sod=0.4'	880
Refusal (Hand auger)	870

SOIL PROFILE

13
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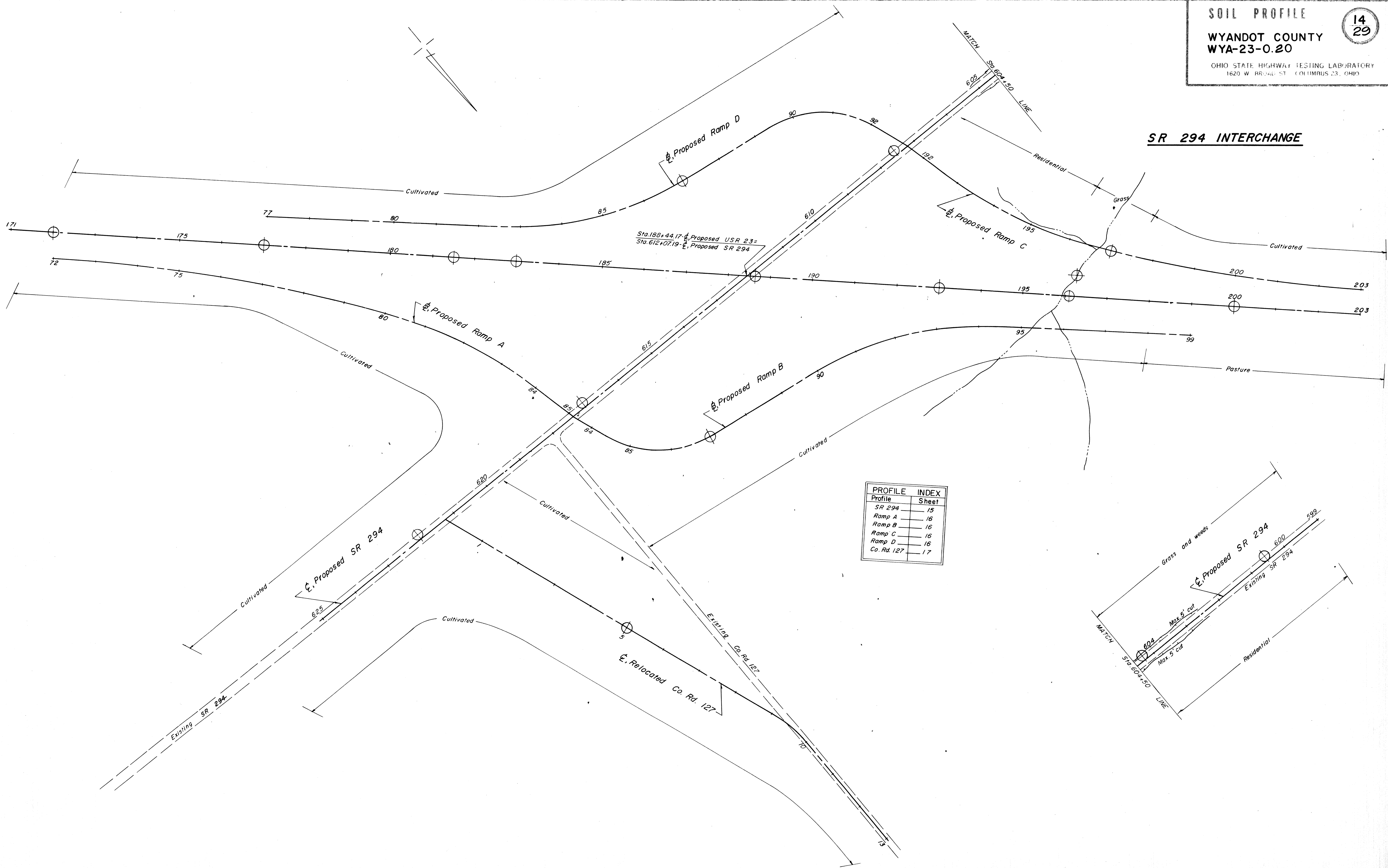
WYANDOT COUNTY
WYA-23-0.20

OHIO STATE HIGHWAY TESTING LABORATORY
1620 W. BROAD ST. COLUMBUS 23, OHIO

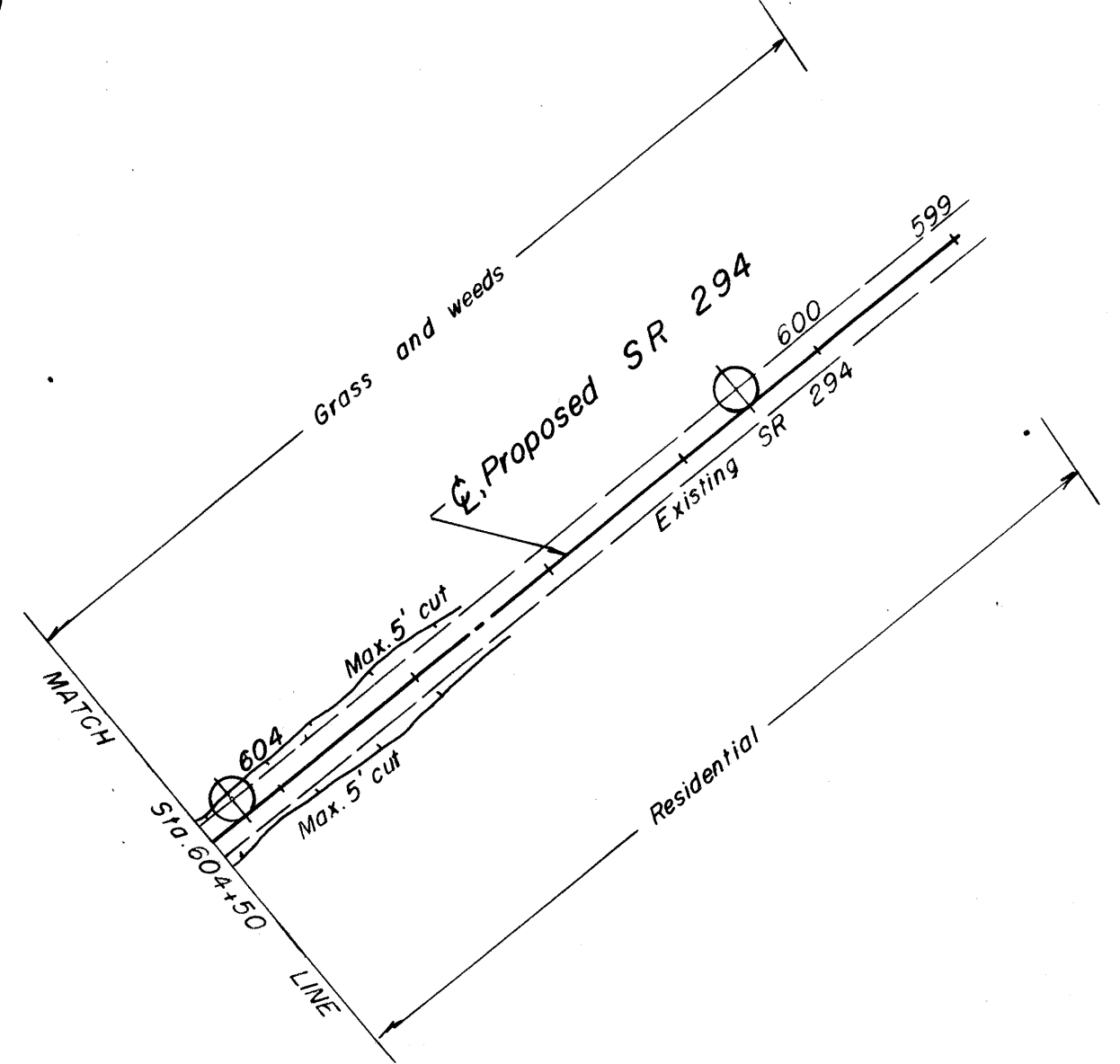


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SR 294 INTERCHANGE



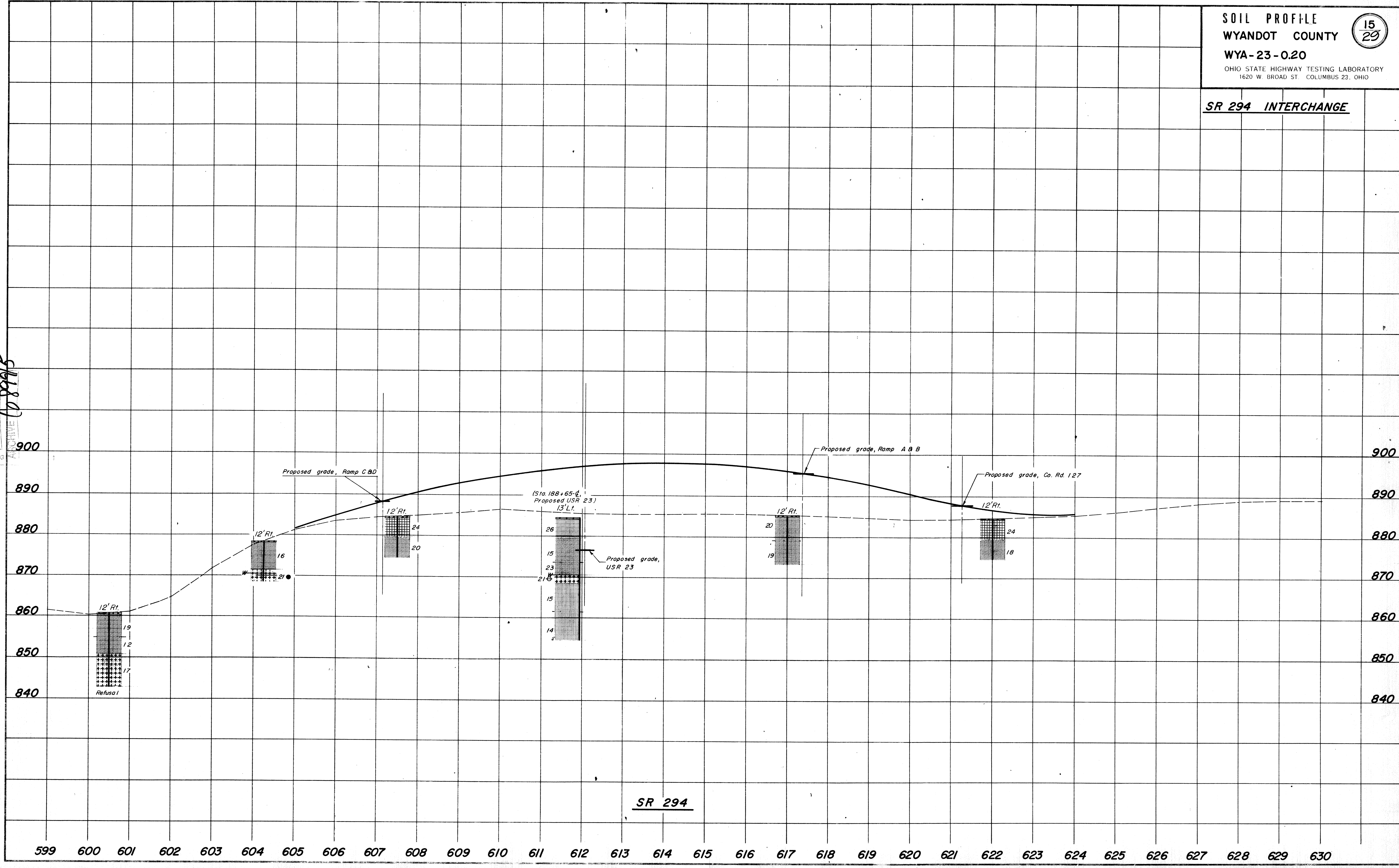
Profile	Sheet
SR 294	15
Ramp A	16
Ramp B	16
Ramp C	16
Ramp D	16
Co. Rd. 127	17



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 DISTRICT 1
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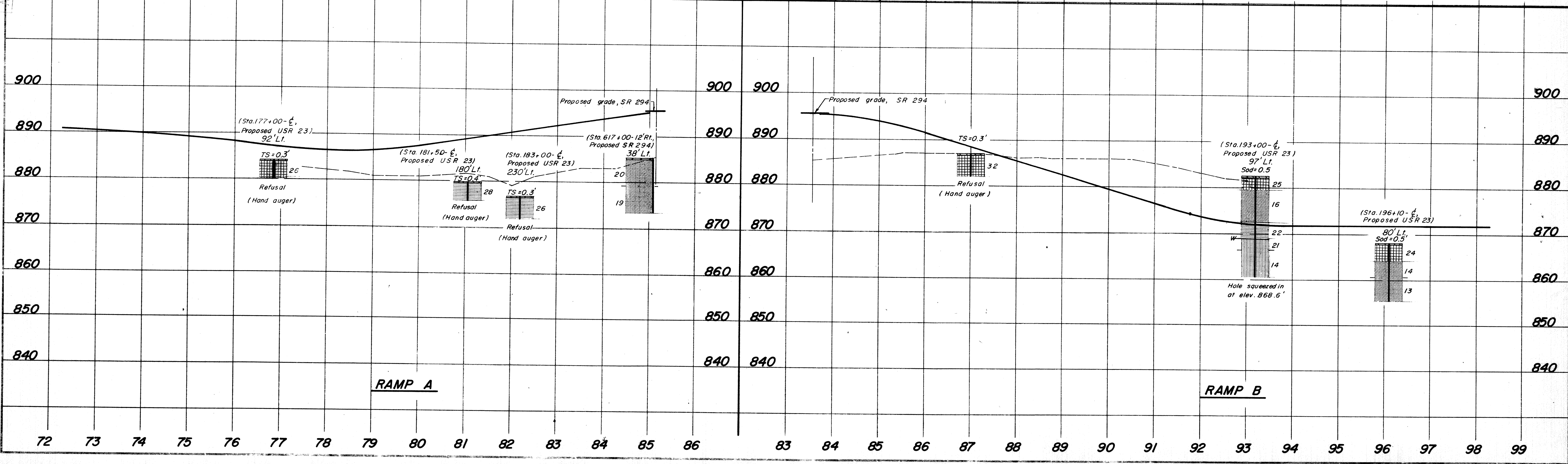
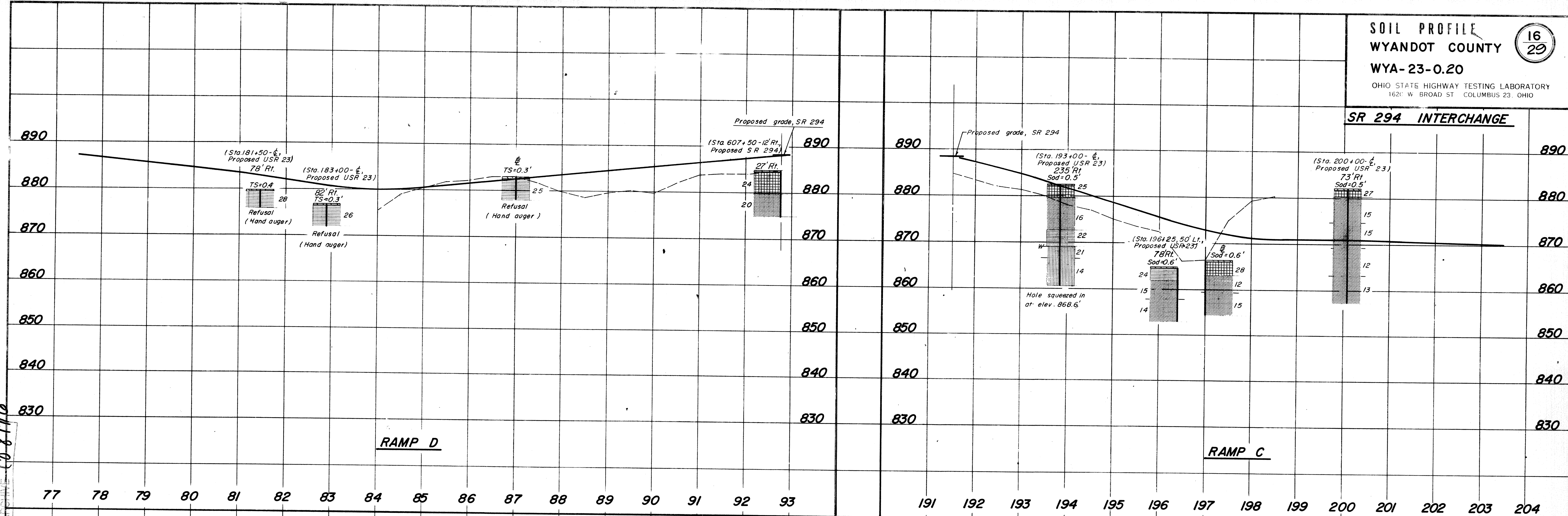
SR 294 INTERCHANGE

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DISTRICT
ARCHIVE
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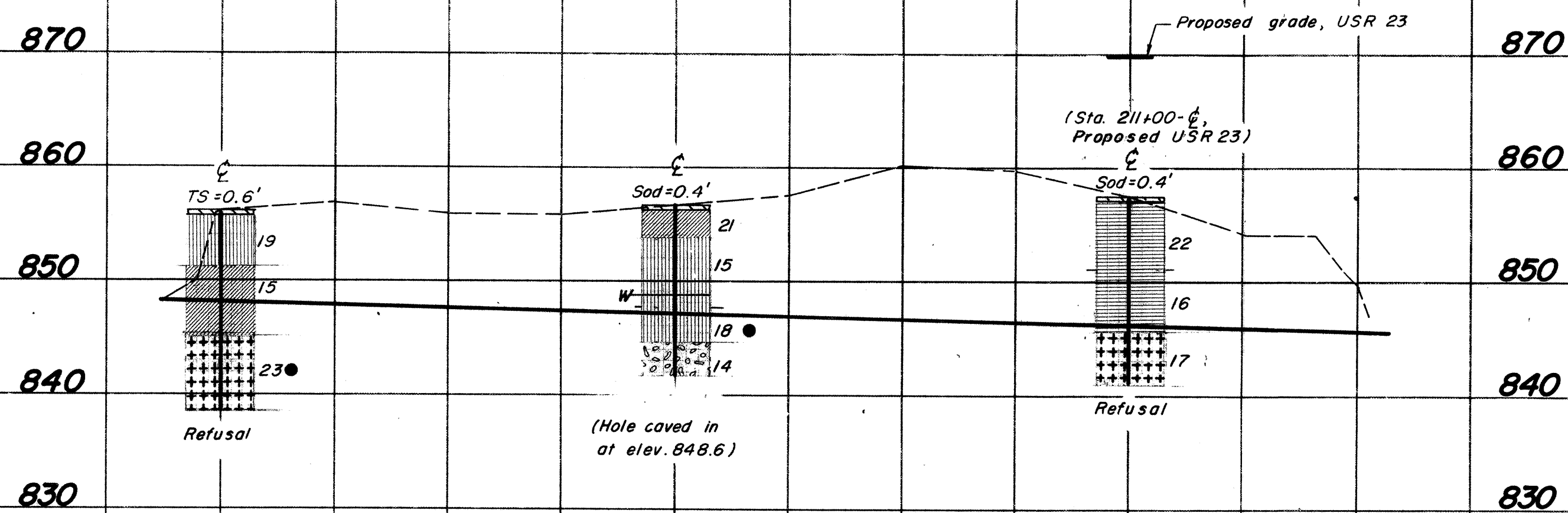


SR 294

SR 294 INTERCHANGE

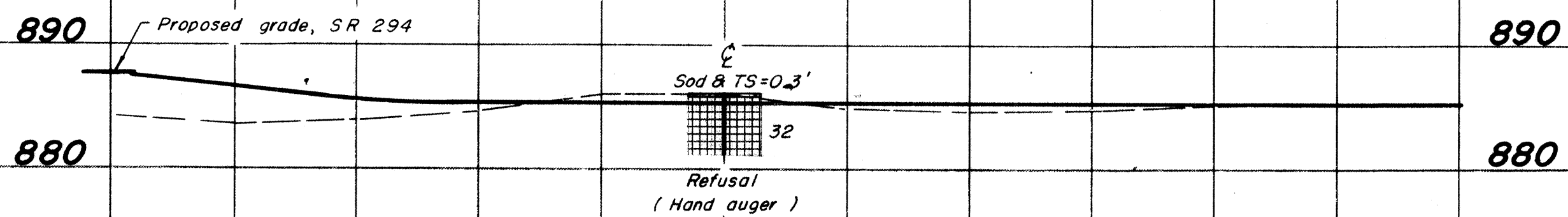


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 DATE
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LITTLE SANDUSKY RIVER RELOCATION

11 12 13 14 15 16 17 18 19 20 21 22 23



CO. RD. 127

0 1 2 3 4 5 6 7 8 9 10 11

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 ARCHIVE
 0089917

GEOLOGY OF THE SITE

The structure site is located on the flat, glaciated Mississippi Valley Plain, on the flood plain of Little Sandusky River, where glacial drift, found to be about 20 feet deep, overlies dolomite bedrock, of the Monroe Series.

EXPLORATION

The exploration consisted of three drive sample-core borings made between December 23 and 30, 1964, and eight drive rod penetration tests, made on December 9 and 10, 1964.

INVESTIGATIONAL FINDINGS








Borings disclosed that bedrock encountered at 19 and 20-foot depths, between elevations 839 to 835 feet, is overlain by moist to wet, generally medium-dense and very stiff gravels, sands, silts, and clays.

Rod soundings met increasing penetration resistance with increasing depth and met refusal or near-refusal to penetration at 17 to 20-foot depths, elevations 841 to 836 feet, considered to be on or slightly above bedrock surface.

On the basis of tests, bedrock surface is considered to be sloping gently downward from the rear to the forward portion of the structure site between elevations 839 and 835 feet.










Free water was noted in the rod sounding holes between 5 and 12-foot depths, elevations 852 and 846 feet.







LEGEND

-  Auger Boring Location - Plan View
-  Press and/or Drive Sample and/or Core Boring Location - Plan View
-  Drive Rod Penetration Resistance Sounding Location - Plan View
-  Capped Pile
-  Footing
-  Footing on Pile
-  Tip of Rock

SYMBOLS OF ROCK TYPES

-  Coal
-  Weathered Indurated Clay
-  Indurated Clay
-  Weathered Shale
-  Shale

-  Horizontal Bar on Boring Log Indicates the Depth the Sample Was Taken.
-  X/Y Figures Beside the Boring Log in Profile Indicate the Number of Blows For Standard Penetration Test.
X = Number of Blows for First 6 Inches.
Y = Number of Blows for Second 6 Inches.
-  Drive Rod Penetration Resistance Sounding Log - Profile.
-  Casing
-  Resistance "R" < 10,000 lbs.
-  Resistance "R" > 10,000 lbs.
-  Z Indicates Final Measurement of Penetration, in Inches.
-  W Indicates Free Water Elevation.
-  V Indicates Static Water Elevation.

-  Weathered Sandstone
-  Sandstone
-  Leached Dolomite
-  Granite
-  Leached Limestone
-  Limestone

GENERAL INFORMATION

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with post performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

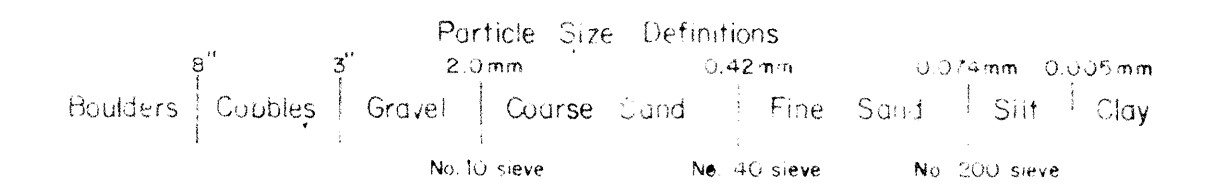
Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D. 1-3/8" I.D. sampler, at 2-1/2 and/or 5-foot depth intervals, driven by means of a 140-pound drop-hammer, with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D. 1-3/8" I.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on laboratory tests and the Casagrande AC classification system - and gradation, plasticity, and moisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.



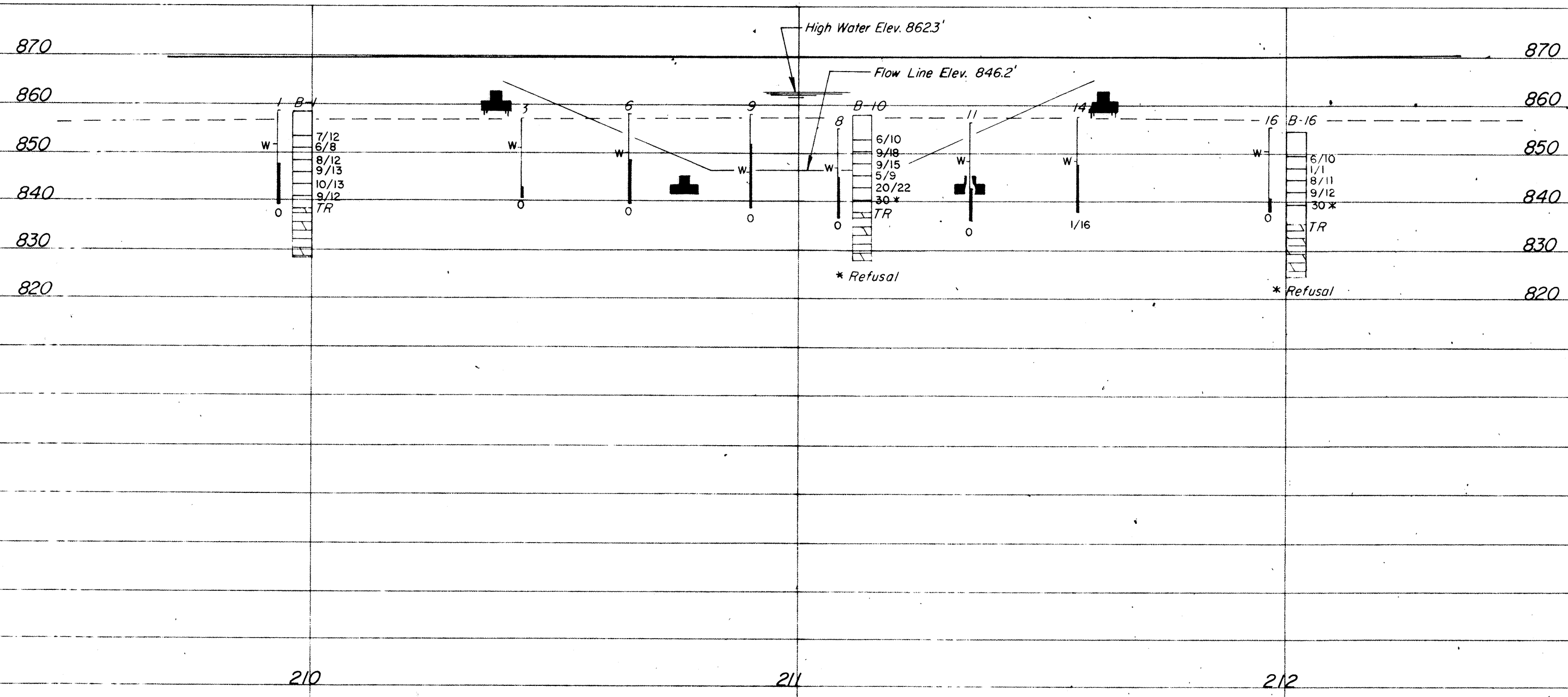
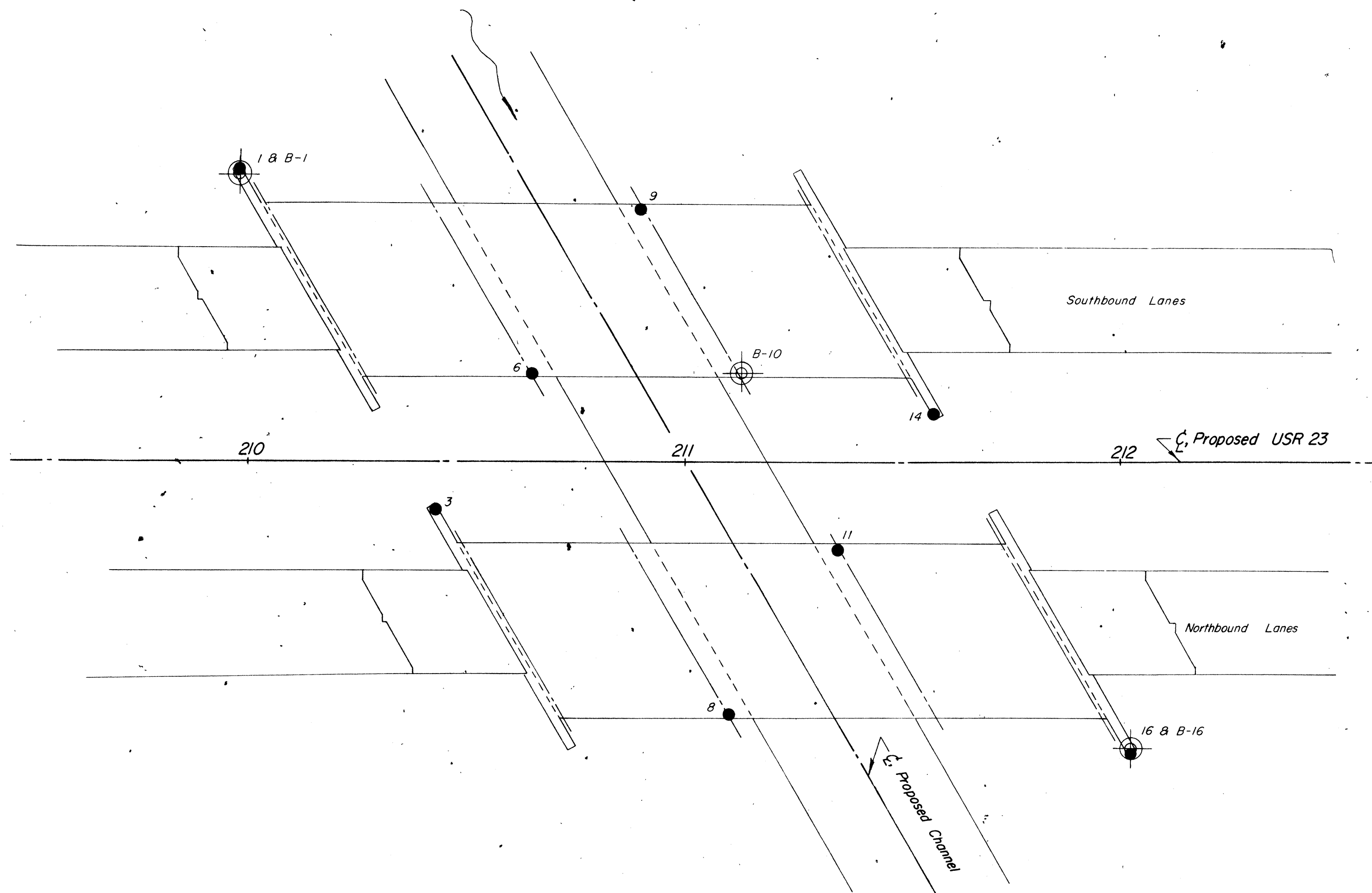
NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design criteria for the project. The State of Ohio does not guarantee the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO STATE HIGHWAY
TESTING LABORATORY
1620 WEST BROAD STREET, COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
BRIDGE NO. WYA-23-0400
OVER RELOCATED LITTLE SANDUSKY RIVER
SEC. WYA-23-0.20

CHECKED BY: L.N.L. REVIEWED BY: R.D.R. DATE: 1/14/65

SCANNED
JUL 15 2011
NOL00000
DISTRICT 1
ARCHIVE Co 8918



SCANNED
 DATE: 5/27/88
 PROJECT: 19
 ARCHIVE
 61688
 608999

OHIO STATE HIGHWAY TESTING LABORATORY 1620 WEST BROAD ST., COLUMBUS 23, OHIO			
STRUCTURE FOUNDATION INVESTIGATION BRIDGE NO. WYA-23-0400 OVER RELOCATED LITTLE SANDUSKY RIVER SEC. WYA-23-0.20			
PLAN AND PROFILE			
DRAWN BY	CHECKED BY	REVIEWED BY	DATE
R.L.C.	L.N.L.	R.D.R.	1/14/65

SCALE: 1"=20'

LOG OF BORING

Date Started 12-29-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-29-64 Casing Length 20' Dia. 3 1/2"
 Boring No. B-1 Station & Offset 209+08, 66' 14" (REAR ABUTMENT) Surface Elev. 858.6'

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample No.	Physical Characteristics							SHTL Class.		
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	L.L.	P.I.		W.C.	
858.6	0															
853.6	5	7/32			Brown Clayey Silt	1	0	5	14	37	44	30	10	19		
851.1	7.5	6/8			Brown and Gray Silt	2	0	5	12	37	46	24	6	16		
848.6	10	8/12			Gray Gravelly Silt	3	30	2	5	21	42	30	9	21		
846.1	12.5	9/13			Gray Sandy Silt	4	0	2	21	45	32	NP	NP	22		
843.6	15	10/13			Gray Silt	5	0	3	9	46	42	NP	NP	20		
841.1	17.5	9/12			Gray Silt	6	0	2	9	45	44	NP	NP	8		
838.6	20															
	22				TOP OF ROCK											
	24		4.6	0.4	Dolomite, gray, hard, broken with thin shale stringers. Core Loss 12%.											
	26															
	28		4.2	0.8												
828.6	30				BOTTOM OF BORING											

LOG OF BORING

Date Started 12-28-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-28-64 Casing Length 20' Dia. 3 1/2"
 Boring No. B-10 Station & Offset 211+13, 20' 14" (FORWARD PIER) Surface Elev. 857.9'

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample No.	Physical Characteristics							SHTL Class.		
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	L.L.	P.I.		W.C.	
857.9	0															
852.9	5	6/10			Brown Sandy Clay, Trace of Roots	1			V	I	S	UAL	PI	19	24	
850.4	7.5	9/18			Brown and Gray Sandy Clay	2	0	6	23	18	53	34	11	19		
847.9	10	9/15			Gray Clayey Silt	3	0	3	5	36	56	35	10	19		
845.4	12.5	5/9			Gray Clayey Silt	4	0	2	4	38	56	26	8	18		
842.9	15	20/22			Gray Gravelly Silt	5	33	3	4	34	26	24	5	18		
840.4	17.5	30* (0.3')			Gray Silty Sandy Gravel	6			V	I	S	U	A	L	10	
837.9	20															
	22				TOP OF ROCK											
	24		5.0	0.0												
	26				Dolomite, gray, hard, jointed with shale stringers. No core loss.											
	28															
827.9	30				BOTTOM OF BORING											

*Refusal

LOG OF BORING

Date Started 12-29-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-30-64 Casing Length 19' Dia. 3 1/2"
 Boring No. B-16 Station & Offset 212+02, 66' 14" (FORWARD ABUTMENT) Surface Elev. 854.4'

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample No.	Physical Characteristics							SHTL Class.		
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	L.L.	P.I.		W.C.	
854.4	0															
849.4	5	6/10			Brown Silty Sandy Gravel	1	45	16	16	19	28	4	8			
846.9	7.5	1/1			Gray Sandy Silt	2	0	8	21	35	36	21	6	22		
844.4	10	8/11			Gray Silty Sand	3	14	10	45	19	12	NP	NP	18		
841.9	12.5	9/12			Gray Silty Sand	4	0	3	74	10	13	NP	NP	22		
839.4	15	30* (0.4')			Gray Silty Sandy Gravel	5	65	13	11	-	11	NP	NP	6		
836.9	17.5				No Sample Recovered - Stone Fragments (Driller's Log)				V	I	S	U	A	L		
834.4	20		1.0	0.0												
	22				TOP OF ROCK											
	24		5.0	0.0												
	26				Dolomite, gray, hard, jointed with few shale stringers. Core loss 2%.											
	28		4.8	0.2												
824.4	30				BOTTOM OF BORING											

*Refusal

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 0089980

OHIO STATE HIGHWAY TESTING LABORATORY 1620 WEST BROAD ST. COLUMBUS 23, OHIO			
STRUCTURE FOUNDATION INVESTIGATION BRIDGE NO. WYA-23-0400 OVER RELOCATED LITTLE SANDUSKY RIVER SEC. WYA-23-020			
BORING DATA			
TYPED BY B.J.M.	CHECKED BY R.C.	REVIEWED BY G.P.H.	DATE 1/14/65

Test Location No. 1
Station & Offset 209+98.67 LT
REAR ABUTMENT
Surface Elev. 858.6' Water Elev. 851.6'

Test Location No. 3
Station & Offset 210+43.11 RT
REAR ABUTMENT
Surface Elev. 857.1' Water Elev. 851.1'

Test Location No. 6
Station & Offset 210+65.20 LT
REAR PIER
Surface Elev. 858.0' Water Elev. 850.0'

Test Location No. 8
Station & Offset 211+10.58 RT
REAR PIER
Surface Elev. 855.1' Water Elev. 847.1'

Test Location No. 9
Station & Offset 210+90.58 LT
FORWARD PIER
Surface Elev. 858.0' Water Elev. 846.0'

Test Location No. 11
Station & Offset 211+35.20 RT
FORWARD PIER
Surface Elev. 856.2' Water Elev. 848.2'

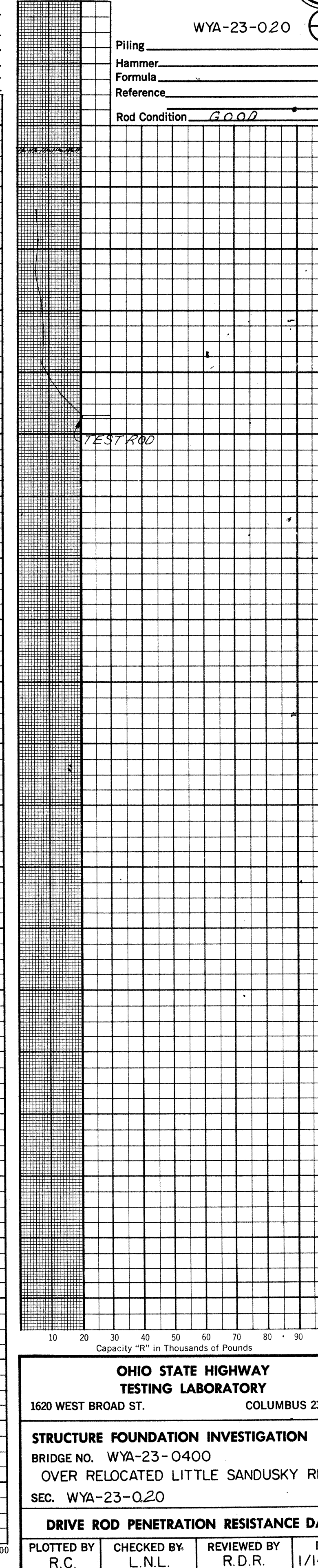
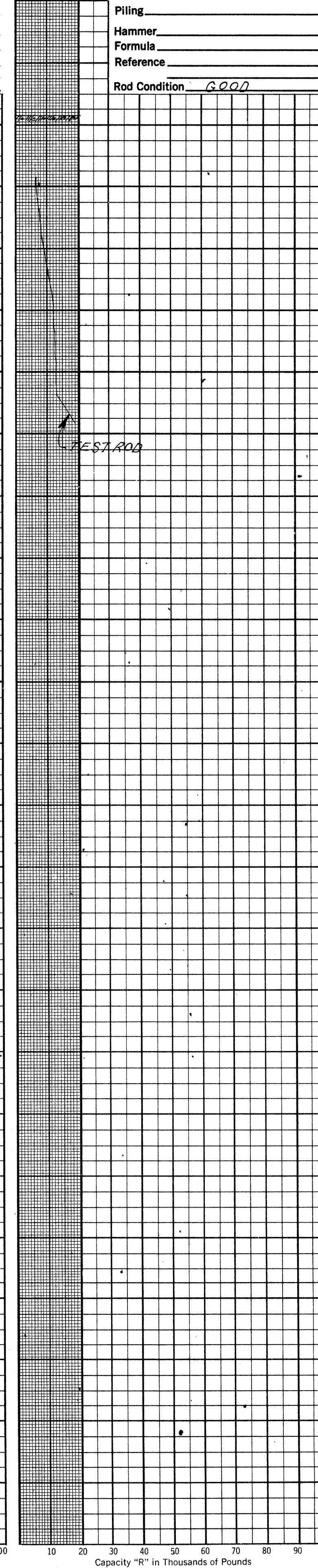
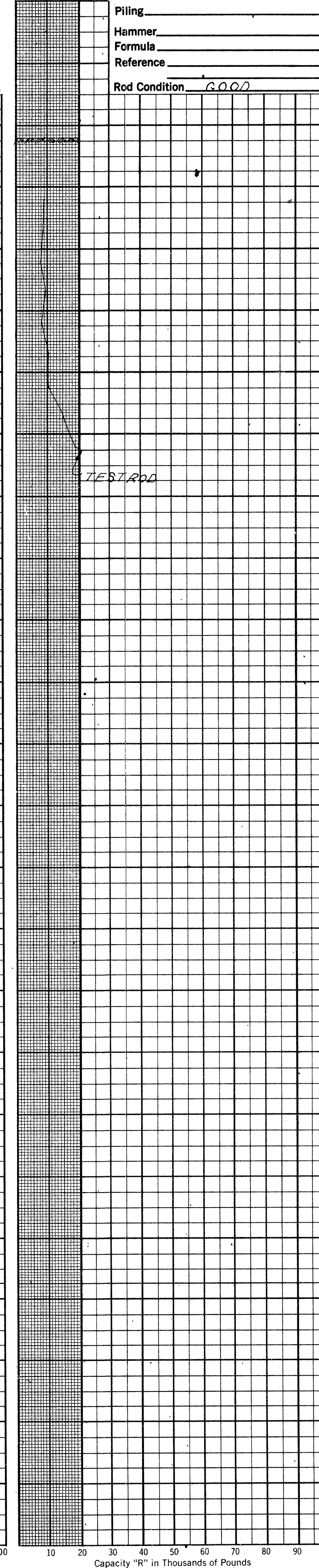
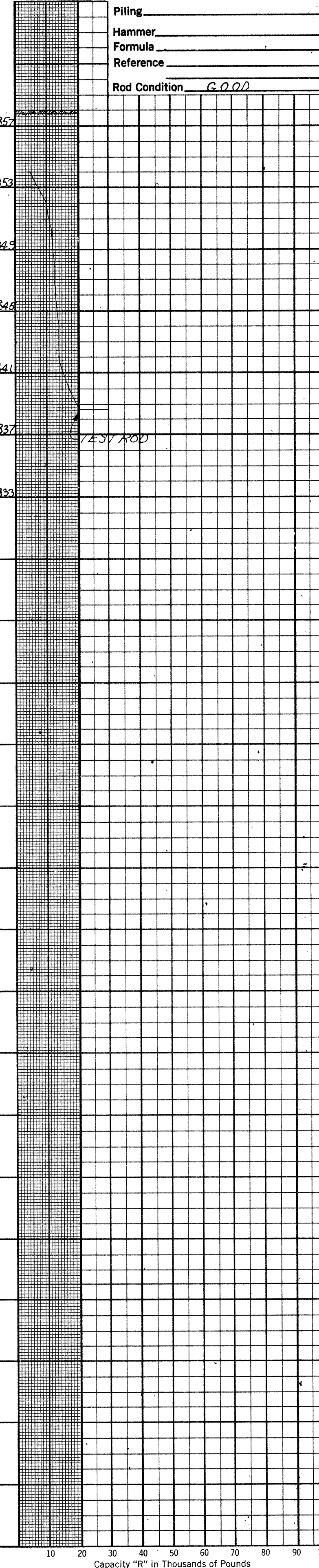
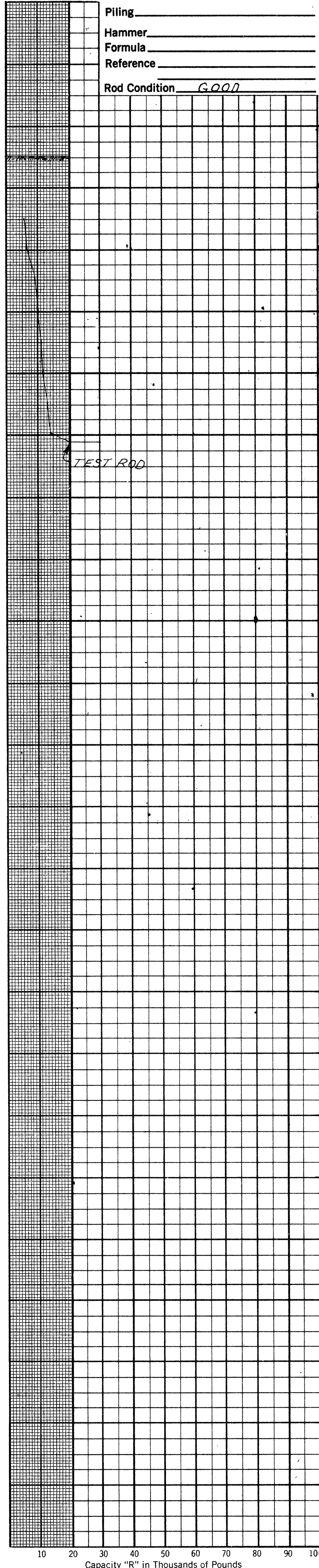
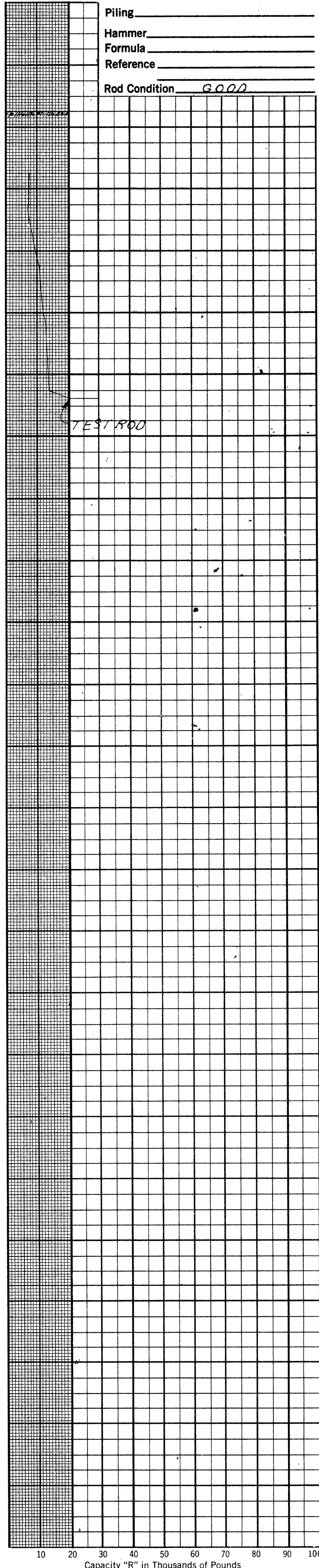
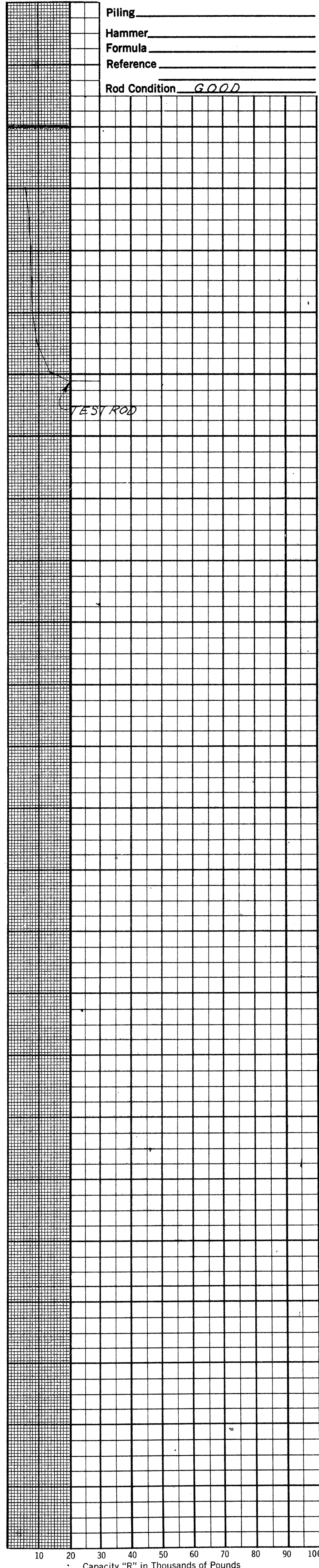
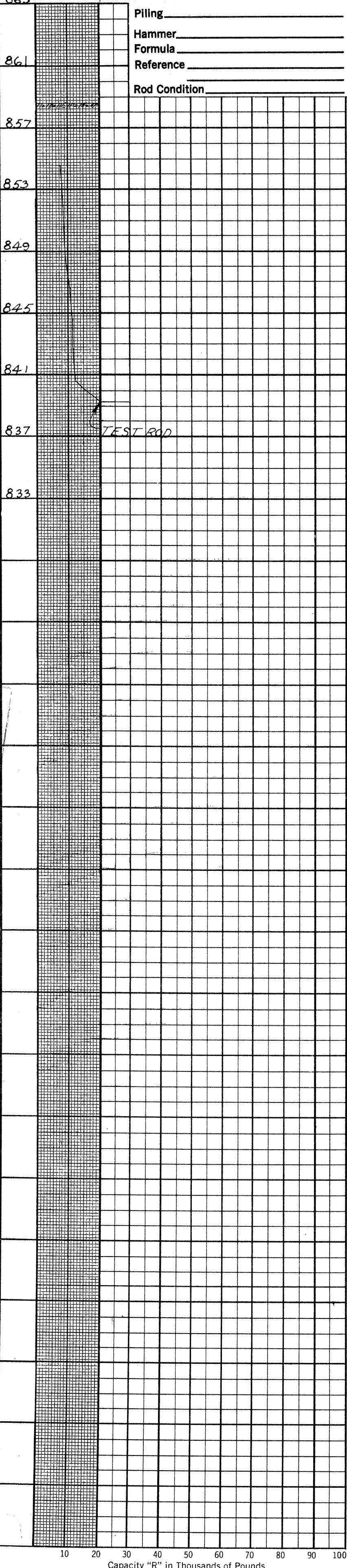
Test Location No. 14
Station & Offset 211+57.11 LT
FORWARD ABUTMENT
Surface Elev. 857.6' Water Elev. 848.6'

Test Location No. 16
Station & Offset 212+02.67 RT
FORWARD ABUTMENT
Surface Elev. 855.6' Water Elev. 850.6'

21
20
4
4

WYA-23-020

Piling _____
Hammer _____
Formula _____
Reference _____
Rod Condition GOOD



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OHIO STATE HIGHWAY TESTING LABORATORY
1620 WEST BROAD ST. COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
BRIDGE NO. WYA-23-0400
OVER RELOCATED LITTLE SANDUSKY RIVER
SEC. WYA-23-020

DRIVE ROD PENETRATION RESISTANCE DATA

PLOTTED BY R.C.	CHECKED BY L.N.L.	REVIEWED BY R.D.R.	DATE 1/14/65
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GEOLOGY OF THE SITE

The structure site is located on the flood plain of Honey Run, on the relatively flat, glaciated upland plain, in an area where moderately deep glacial deposits overlie dolomite bedrock, of Devonian age.

EXPLORATION

The exploration consisted of two drive sample-core borings and four drive rod penetration tests, made between December 8 and 22, 1964.








INVESTIGATIONAL FINDINGS

Borings disclosed medium-stiff gravelly clay and loose to very dense sands, silts, gravels, and boulders overlie bedrock surface, encountered in boring B-2 at 45-foot depth, elevation 838 feet. Boring B-2 was terminated at 55-foot depth, elevation 828 feet, after penetrating 10 feet below bedrock surface. Boring B-7 was terminated at 45-foot depth, elevation 837 feet, in broken rock, probably bedrock.







The rod soundings met with increasing resistance to penetration with increase in depth and were terminated due to refusal or near refusal to penetration at 31 and 36-foot depths, elevations 852 to 847 feet, considered to be in sand, gravel, and boulders, as revealed by the borings.










Free water was encountered in the rod sounding holes between elevations 879 and 876 feet.







LEGEND

-  Auger Boring Location - Plan View.
-  Press and/or Drive Sample and/or Core Boring Location - Plan View.
-  Drive Rod Penetration Resistance Sounding Location - Plan View.
-  Capped Pile
-  Footing
-  Footing on Pile
-  Top of Rock

SYMBOLS OF ROCK TYPES

-  Coal
-  Weathered Indurated Clay
-  Indurated Clay
-  Weathered Shale
-  Shale
- 

-  Horizontal Bar on Boring Log Indicates the Depth the Sample Was Taken.
-  Figures Beside the Boring Log in Profile Indicate the Number of Blows For Standard Penetration Test.
X = Number of Blows for First 6 Inches.
Y = Number of Blows for Second 6 Inches.
-  Drive Rod Penetration Resistance Sounding Log - Profile.
-  Casing
-  Resistance " R " $< 10,000$ lbs.
-  Resistance " R " $\ge 10,000$ lbs.
-  Indicates Final Measurement of Penetration, in Inches.
-  Indicates Free Water Elevation.
-  Indicates Static Water Elevation.

-  Weathered Sandstone
-  Sandstone
-  Leached Dolomite
-  Dolomite
-  Leached Limestone
-  Limestone

GENERAL INFORMATION

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with past performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

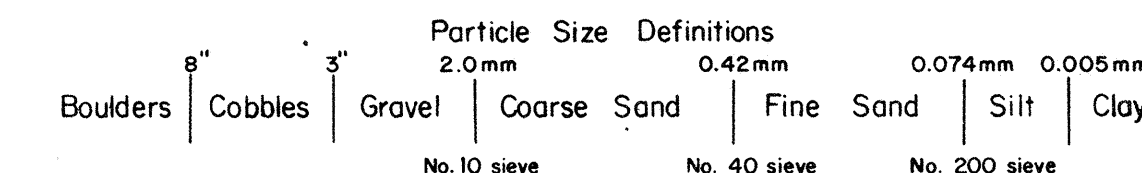
Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. sampler, at 2-1/2 and/or 5-foot depth intervals, driven by means of a 140-pound drop-hammer, with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on laboratory tests and the Casagrande AC classification system - and gradation, plasticity, and moisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.



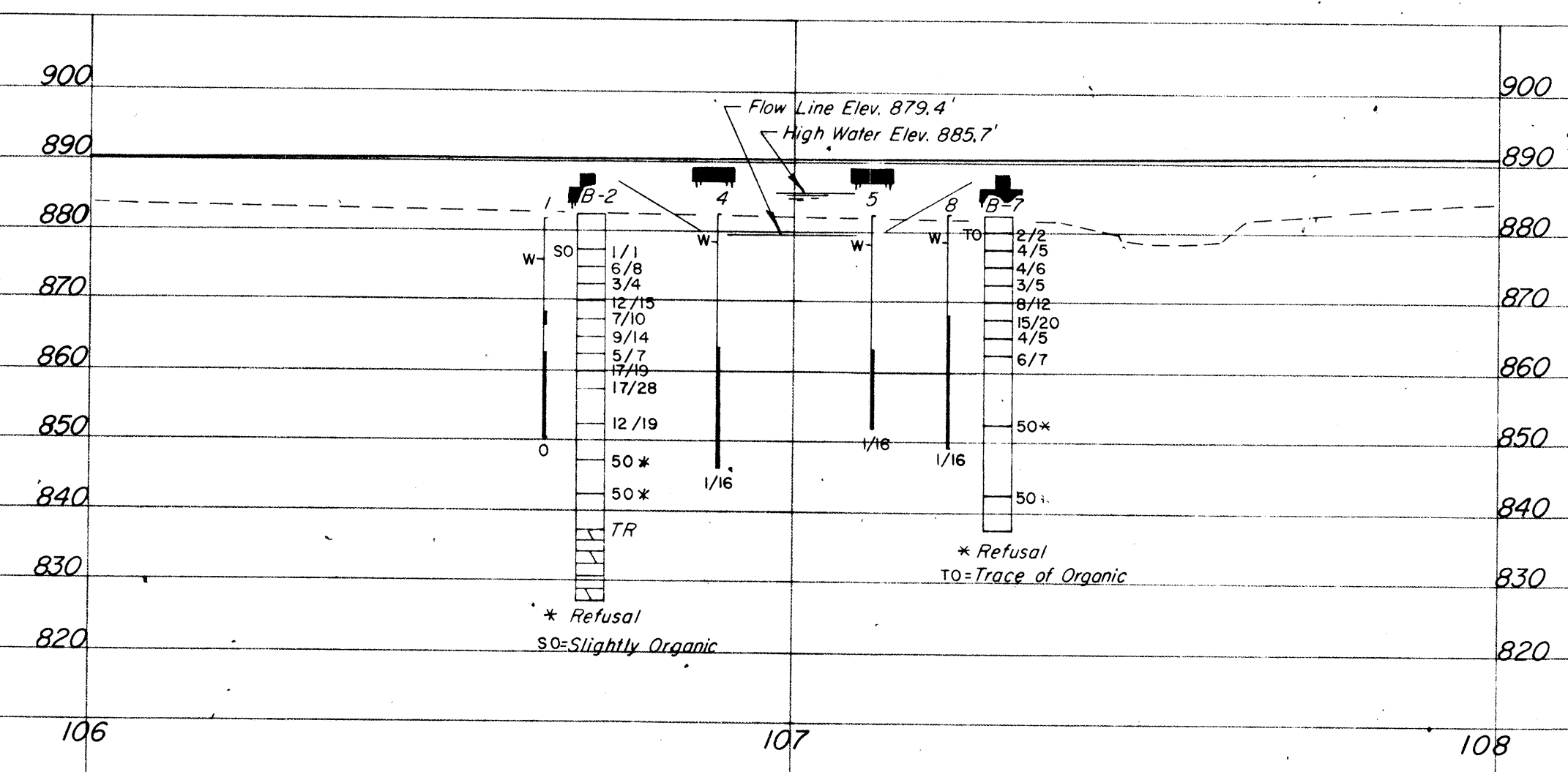
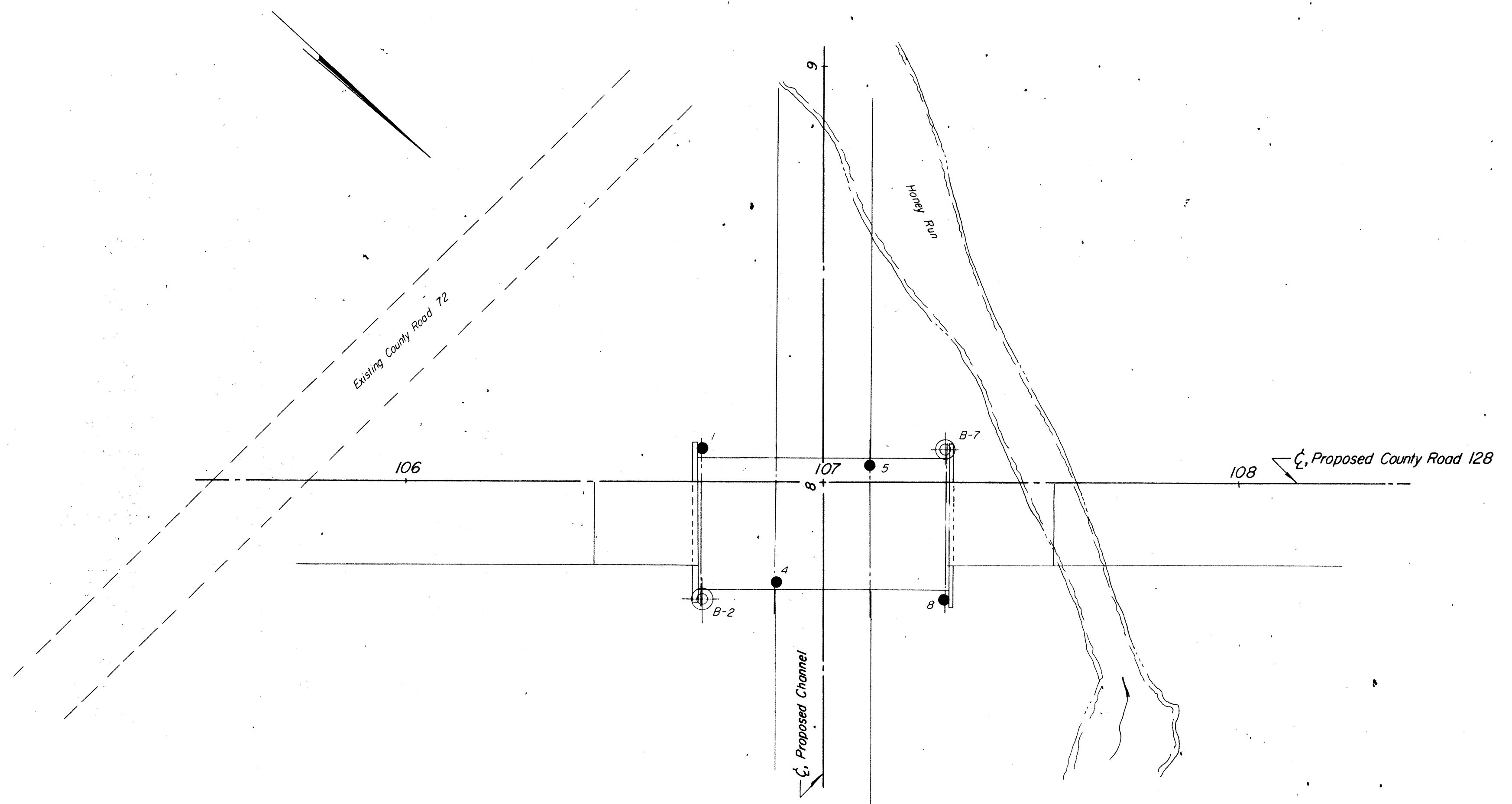
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 DISTRICT 1
 08/19/22

NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design controls for the project. The State of Ohio does not guarantee the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO STATE HIGHWAY
TESTING LABORATORY
1620 WEST BROAD STREET, COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
BRIDGE NO. WYA-23-
RELOCATED C.R. 128 OVER HONEY RUN
SEC. WYA-23-0.20

CHECKED BY R. H. P.	REVIEWED BY R. D. R.	DATE 1-12-65
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OHIO STATE HIGHWAY
 TESTING LABORATORY
 1620 WEST BROAD ST., COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
 BRIDGE NO. WYA-23-
 RELOCATED C.R. 128 OVER HONEY RUN
 SEC. WYA-23-020

PLAN AND PROFILE

DRAWN BY R. L. C.	CHECKED BY R. H. P.	REVIEWED BY R. D. R.	DATE 1-12-65
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SCALE: 1"=20'

SCANNED
 DISTRICT 1
 ARCHIVE 089923

LOG OF BORING
 Date Started 12-18-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-22-64 Casing Length 45' Dia. 3 1/2"
 Boring No. B-2 Station & Offset 106+71, 28' Rt. (REAR ABUTMENT) Surface Elev. 882.5'

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample No.	Physical Characteristics								SHTL Class.			
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	LL	PL	W.C.				
882.5	0																	
	2																	
	4																	
877.5	6	1/1			Gray Sandy Silt, Slightly Organic	1	0	5	44	30	21	28	8	33				
875.0	8	6/8			Brown and Gray Silt	2	0	0	5	59	36	NP	NP	22				
872.5	10	3/4			Brown and Gray Silt	3	0	0	3	67	30	NP	NP	22				
870.0	12																	
	14	12/15			Gray Silty Sandy Gravel	4	45	12	9	15	19	FL	17	16				
867.5	16	7/10			Gray Sandy Gravelly Silt	5	31	5	10	24	30	25	8	15				
865.0	18	9/14			Gray Sandy Gravelly Silt	6	28	15	9	23	25	25	8	17				
862.5	20	5/7			Gray Sandy Silt	7	0	6	22	34	38	23	7	15				
860.0	22	17/19			Gray Sandy Gravelly Silt	8	32	6	13	25	24	22	7	12				
857.5	26	17/28			Gray Sandy Gravelly Silt	9	41	8	9	23	19	25	9	12				
	28																	
852.5	30	12/19			Gray Gravelly Silt	10	32	2	1	43	22	30	10	18				
	32																	
	34																	
847.5	36	50/*			Gray Silty Gravel	11	69	8	5	6	12	NP	NP	5				
	38																	
842.5	40																	
	42	(50*)			Light-Gray Silty Sandy Gravel	12	45	10	10	18	17	NP	NP	15				
	44																	
837.5	46																	
	48		5.0	0.0	TOP OF ROCK Dolomite, gray, hard, with many shale stringers. No Core Loss.													
	50																	
	52																	
	54		5.0	0.0	BOTTOM OF BORING													
827.5																		

LOG OF BORING
 Date Started 12-16-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-17-64 Casing Length 40' Dia. 3 1/2"
 Boring No. B-7 Station & Offset 107+29, 8' Lt. (FORWARD ABUTMENT) Surface Elev. 882.4'

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample No.	Physical Characteristics								SHTL Class.			
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	LL	PL	W.C.				
882.4	0																	
	2																	
879.9	4	2/2			Dark Brown and Gray Gravelly Clay. Trace of Organic	1	45	2	8	20	25	37	16	24				
877.4	6	4/5			Gray and Brown Silt and Clay	2	0	4	9	39	48	33	12	22				
874.9	8	4/6			Gray Silt	3	0	1	6	64	29	24	3	28				
872.4	10																	
	12	3/5			Gray Silt	4	0	2	5	66	27	NP	NP	24				
869.9	14	8/12			Gray Sandy Gravelly Silt	5	32	6	11	25	26	24	8	13				
867.4	16	15/20			Gray Sandy Gravelly Silt	6	V	I	S	U	A	L	21	8	16			
864.9	18	4/5			Gray Sandy Gravelly Silt	7	V	I	S	U	A	L	22	7	12			
862.4	20	6/7			Gray Gravelly Sandy Silt	8	27	3	30	22	18	NP	NP	16				
	22																	
857.4	24				No Sample Recovered-Boulders (Driller's Description)							V	I	S	U	A	L	
	26																	
	28																	
852.4	30	50* (0.2)			Brown Silty Sandy Gravel	9	V	I	S	U	A	L		10				
	32																	
	34																	
847.4	36				No Sample Recovered-Boulders (Driller's Description)							V	I	S	U	A	L	
	38																	
842.4	40																	
	42	50* (0.2)			Gray Broken Dolomitic Limestone	11	V	I	S	U	A	L						
	44																	
837.4					BOTTOM OF BORING	12	V	I	S	U	A	L						

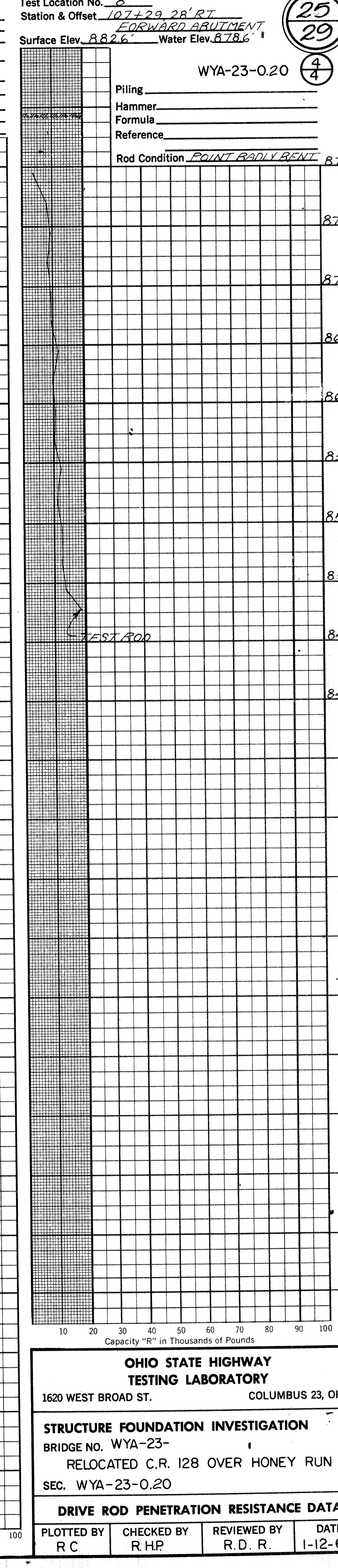
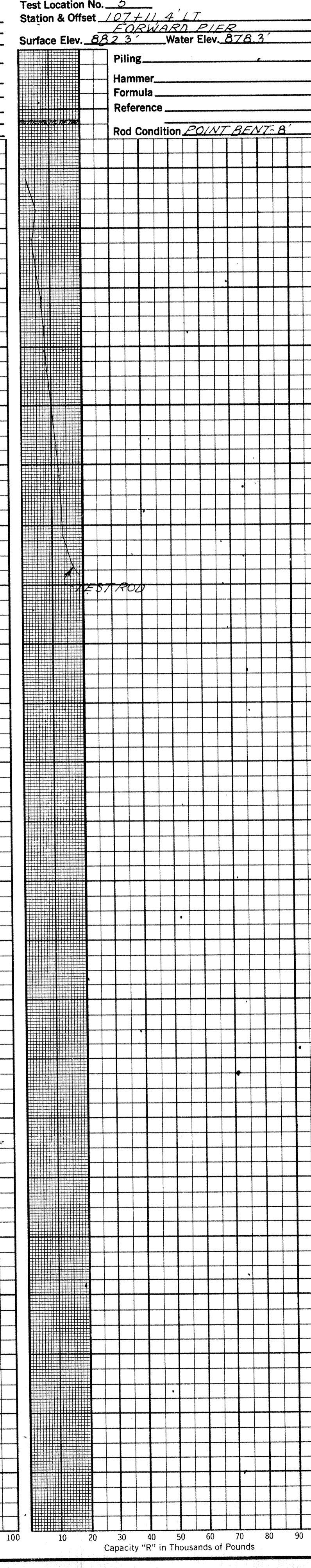
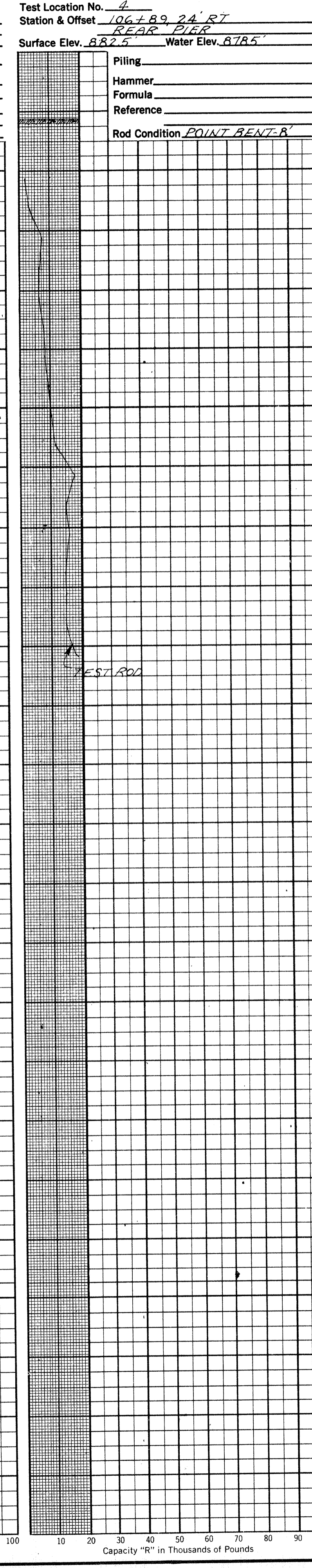
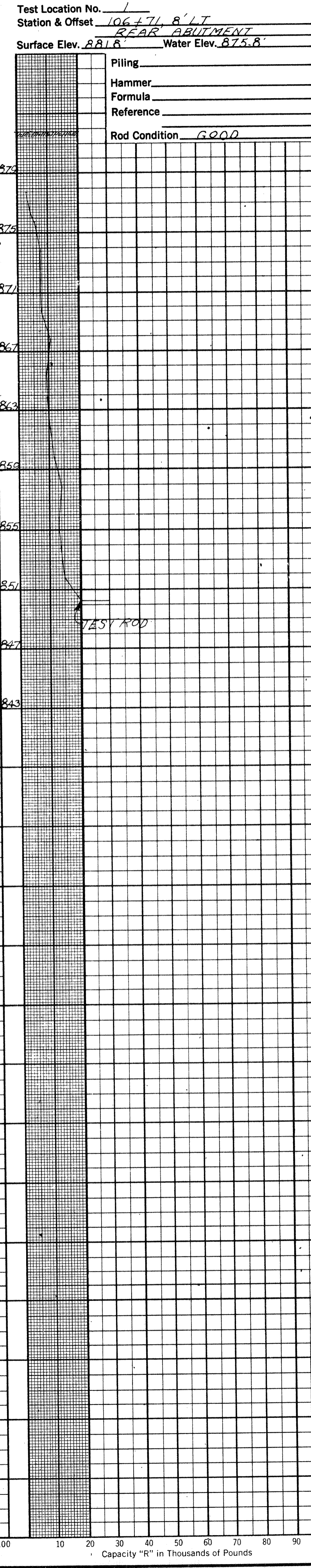
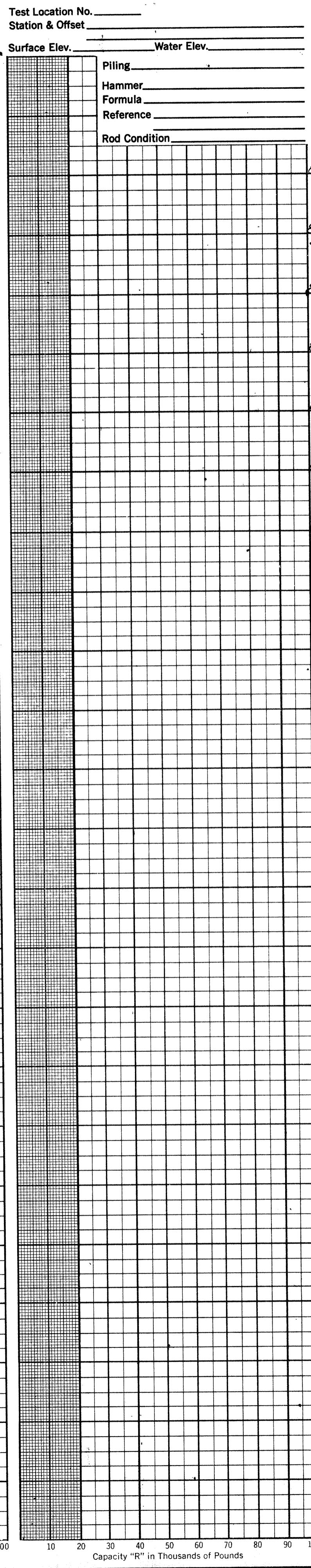
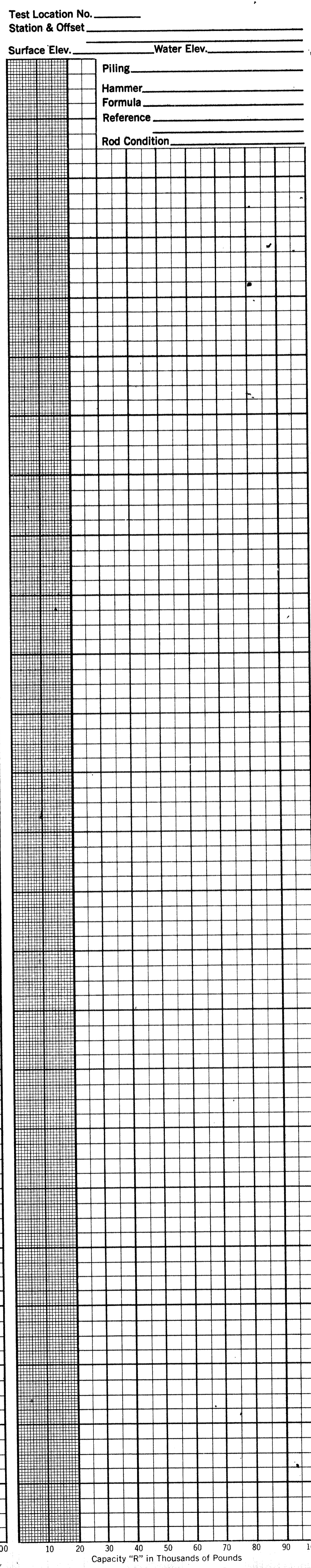
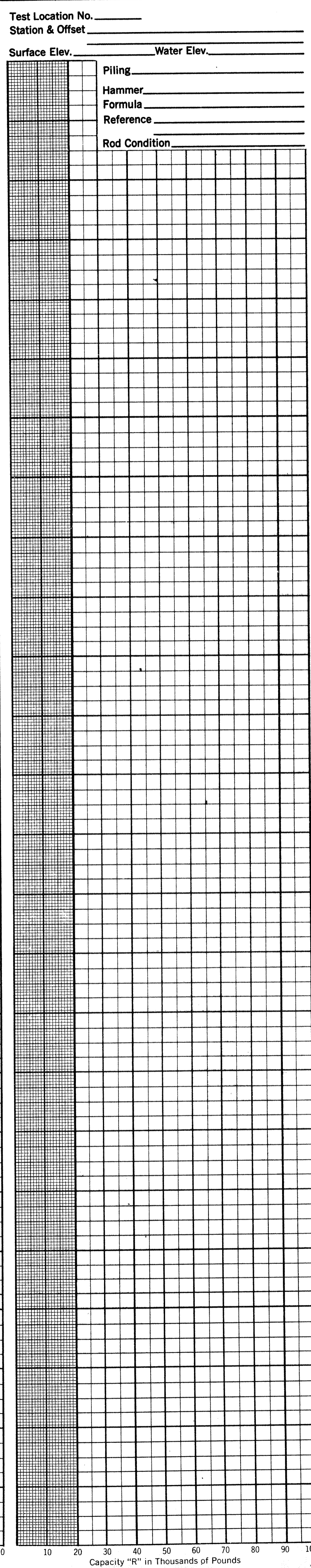
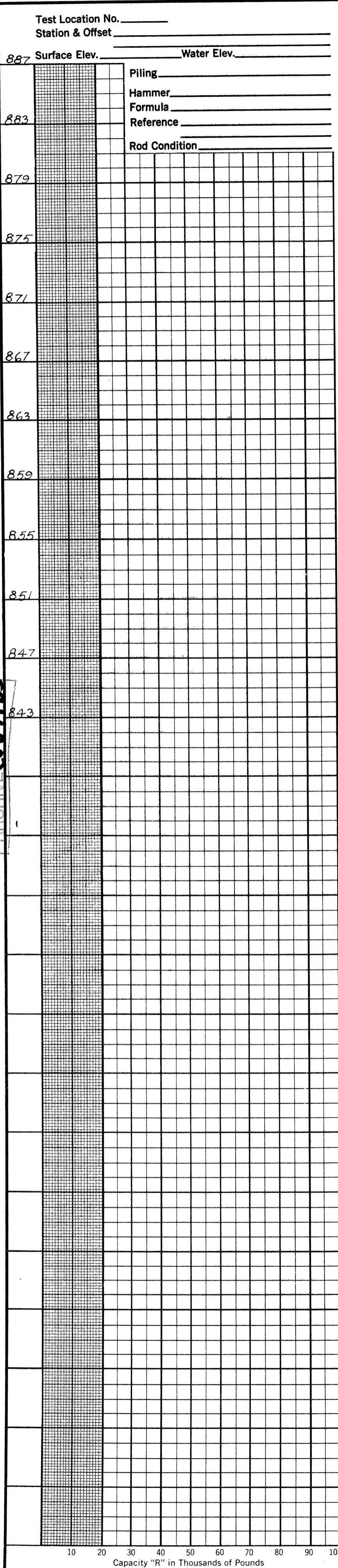
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 DISTRICT 1
 ARCHIVE C089924

OHIO STATE HIGHWAY
 TESTING LABORATORY
 1620 WEST BROAD ST. COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
 BRIDGE NO. WAY-23 -
 RELOCATED CR 128 OVER HONEY
 SEC. WAY-23-0.20

BORING DATA

TYPED BY P.A.S.	CHECKED BY R.C.	REVIEWED BY G.P.H.	DATE 1-12-65
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25
29

WYA-23-0.20
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OHIO STATE HIGHWAY TESTING LABORATORY
1620 WEST BROAD ST. COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
BRIDGE NO. WYA-23-
RELOCATED C.R. 128 OVER HONEY RUN
SEC. WYA-23-0.20

DRIVE ROD PENETRATION RESISTANCE DATA

PLOTTED BY R.C.	CHECKED BY R.H.P.	REVIEWED BY R.D.R.	DATE 1-12-65
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GENERAL INFORMATION

LOCATION OF THE SITE
The structure site is located on the flood plain of Honey Run on the relatively flat, glaciated upland plain, in an area where moderately deep glacial deposits overlie dolomite bedrock, of Devonian age.

EXPLANATION

The exploration consisted of two drive sample-core borings and seven drive rod penetration tests, made between December 2 and 15, 1964.

INVESTIGATIONAL FINDINGS

Borings disclosed medium-stiff gravelly clay and loose to very dense sands, silts, gravels, and boulders to bedrock surface, encountered at 45-foot depth, elevations 836 and 837 feet. The borings were terminated at 55-foot depth, elevations 826 and 827 feet, after penetrating 10 feet of bedrock.

The rod soundings met with increasing resistance to penetration with increase in depth and were terminated due to refusal or near refusal to penetration at 30 to 33-foot depths, elevations 852 to 850 feet, considered to be in sand and gravel, as revealed by the borings.

Free water was encountered in the rod sounding holes between elevations 880 and 876 feet.

LEGEND

- Auger Boring Location - Plan View
- Press and/or Drive Sample and/or Core Boring Location - Plan View
- Drive Rod Penetration Resistance Sounding Location - Plan View
- Capped Pile
- Footing
- Footing on Pile
- Top of Rock

SYMBOLS OF ROCK TYPES

- Coal
- Weathered Indurated Clay
- Indurated Clay
- Weathered Shale
- Shale

- Horizontal Bar on Boring Log Indicates the Depth the Sample Was Taken.
- Figures Beside the Boring Log in Profile Indicate the Number of Blows For Standard Penetration Test.
X = Number of Blows for First 6 Inches.
Y = Number of Blows for Second 6 Inches.
- Drive Rod Penetration Resistance Sounding Log - Profile
- Casing
- Resistance "R" < 10,000 lbs.
- Resistance "R" > 10,000 lbs.
- Indicates Final Measurement of Penetration, in Inches.
- Indicates Free Water Elevation.
- Indicates Static Water Elevation.

- Weathered Sandstone
- Sandstone
- Leached Dolomite
- Dolomite
- Leached Limestone
- Limestone

GENERAL INFORMATION

Drive Rod Penetration Sounding Tests

Drive rod penetration resistance tests constitute driving a 1.315-inch diameter steel rod, with a 45° cone point, into the ground, using a 122-pound drop-hammer with a free fall of five feet. At one or two-foot depth intervals, a measurement is taken to determine the amount of penetration achieved in three hammer drops. This reading is converted to an empirical value for capacity "R", in thousands of pounds (which is a measure of both the point resistance and frictional resistance on the rod), by using charts prepared by the Ohio Department of Highways, Bureau of Bridges, on the basis of correlation study of rod penetration with past performance of pile driving. For interpretation, a graph is prepared by plotting the value "R" against the depth at which the reading was taken, and connecting the plotted points. The curve so obtained reflects the density of subsurface materials in a manner that can be readily compared with data from similar tests at other locations on the structure site. From this comparison, the overall uniformity of subsurface condition may be evaluated.

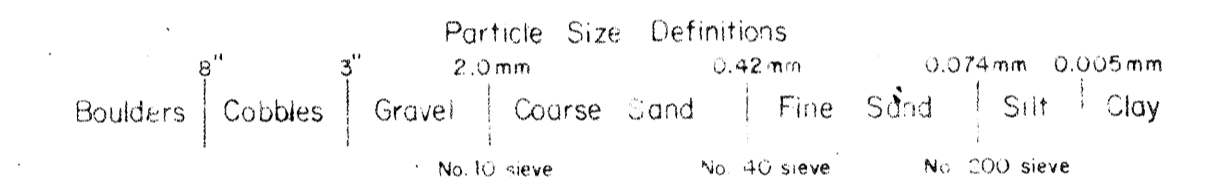
Drive Sample Borings - Drive-Press Sample Borings

Drive sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. sampler, at 2-1/2 and/or 5-foot depth intervals, driven by means of a 140-pound drop hammer, with a free fall of 30 inches. The number of blows required to drive the sampler 12 inches is considered the standard penetration test.

Drive-press sample borings are made by means of a rotary-type drill rig, employing a 2" O.D., 1-3/8" I.D. drive sampler, and 3" O.D. thin-wall press sampler. The press sampler is advanced by continuous uniform pressure, applied by the drill rig.

The boring log sheets show a graphic plot of the information obtained, including depth and elevation of the sample, number of blows for the standard penetration tests in two 6-inch increments, depth of press samples, field sample number, sample description - based on laboratory tests and the Casagrande, AC classification system - and gradation, plasticity, and moisture content determinations. Results of strength and consolidation testing, if performed, appear on separate enclosures.

At depths where materials are bouldery or gravelly to the extent that the sampler can not be driven, a wash sample is procured for visual classification, in order to determine the general character of the material. These samples are not considered sufficiently representative to warrant laboratory testing.



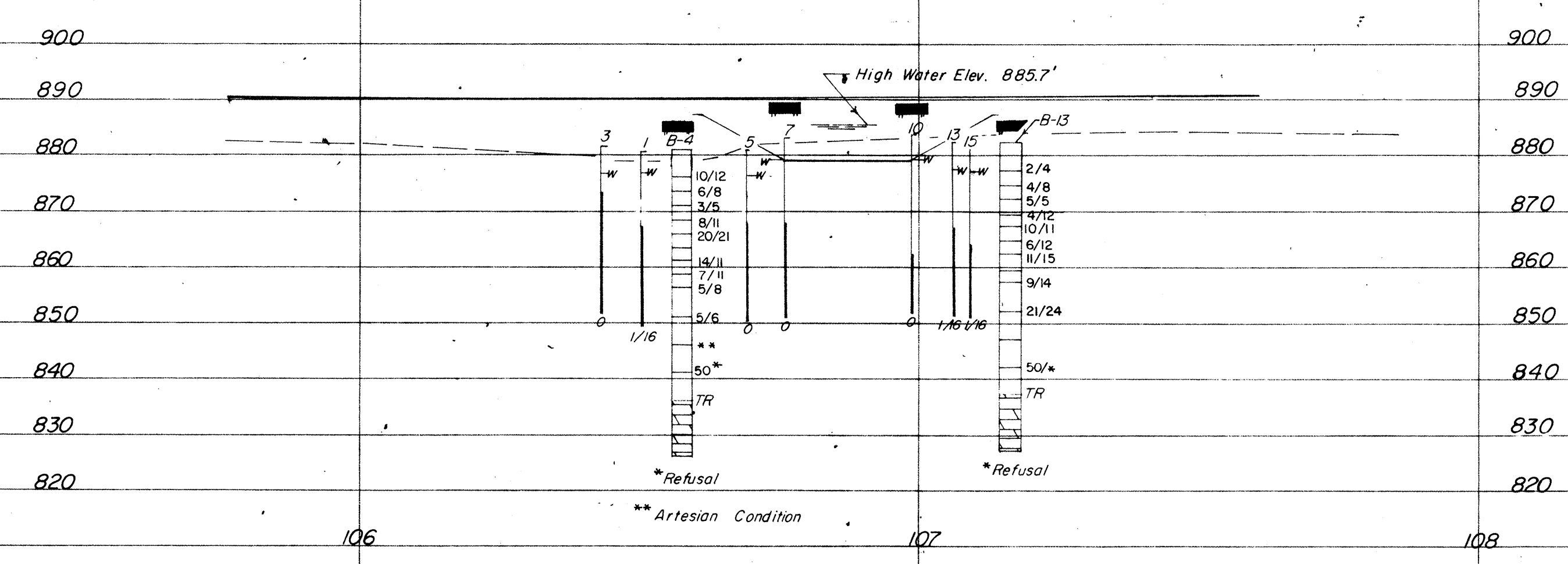
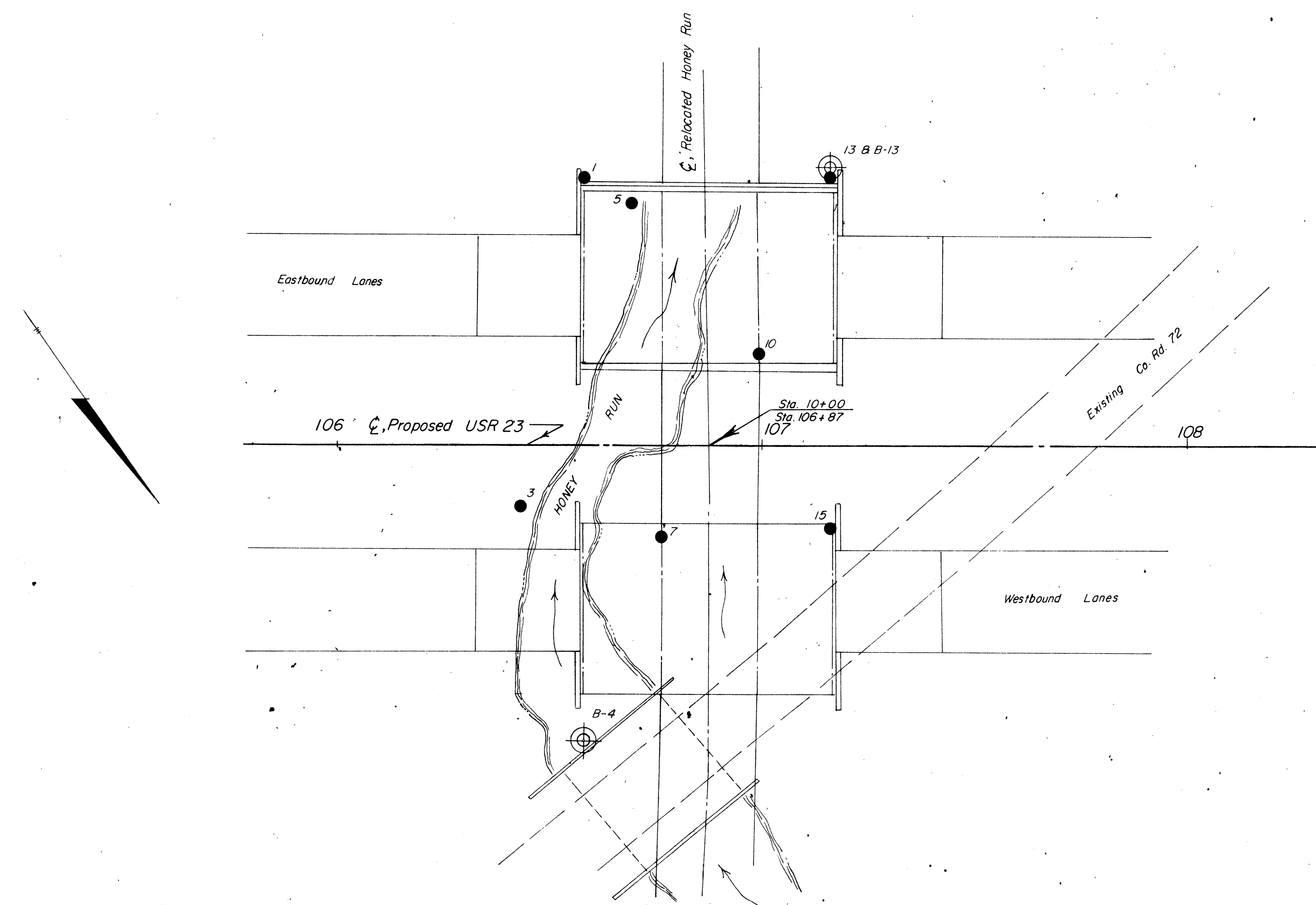
NOTE: Information shown by this subsurface investigation was obtained solely for the use in establishing design criteria for the project. The State of Ohio does not guarantee the accuracy of this data and it is not to be construed as a part of the plans governing construction of the project.

OHIO STATE HIGHWAY
TESTING LABORATORY
1620 WEST BROAD STREET, COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
BRIDGE NO. WYA-23-0202 L/R
OVER RELOC. HONEY RUN
SEC. WYA-23-0.20

CHECKED BY R.H.P. REVIEWED BY G.P.H. DATE 1/6/65

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DISTRICT ARCHIVE 0089920



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 DISTRICT 1
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OHIO STATE HIGHWAY
TESTING LABORATORY
1620 WEST BROAD ST., COLUMBUS 23, OHIO

STRUCTURE FOUNDATION INVESTIGATION
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SEC. WYA-23-0.20

PLAN AND PROFILE

DRAWN BY R.L.F.	CHECKED BY R.H.P.	REVIEWED BY G.P.H.	DATE 1/6/65
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SCALE: 1"=20'

LOG OF BORING

Date Started 12-7-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-10-64 Casing Length 45' Dia. 3 1/2" Surface Elev. 881.0'
 Boring No. B-4 Station & Offset 106+56, 70' R/L (REAR ABUTMENT)

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample no.	Physical Characteristics							SHTL Class.				
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	LL	PI		W.C.			
881.0	0																	
876.0	5	10/12			Gray Sandy Clayey Silt	1	11	4	7	47	31	27	9	16				
873.5	8	6/8			Gray Sandy Silt	2	0	2	28	49	21	NP	NP	19				
871.0	10	3/5			Gray Silt	3	0	1	3	73	23	NP	NP	27				
868.5	12	8/11			Gray Silt	4	0	5	7	55	33	NP	NP	20				
866.0	14	20/21			Gray Sandy Gravelly Silt	5	33	10	16	31	10	NP	NP	16				
863.5	18				Gray Sand (Wash Sample)	6			V	I	S	U	A	L				
861.0	20	14/11			Gray Sandy Gravelly Silt	7	37	7	11	23	22	20	5	15				
858.5	22	7/11			Gray Sandy Gravelly Silt	8	33	7	12	30	18	23	7	18				
856.0	24	5/8			Gray Sandy Gravelly Silt	9	29	11	16	28	16	20	5	11				
851.0	30	5/6			Gray and Brown Silty Sandy Gravel	10	47	8	15	19	11	NP	NP	13				
846.0	34				Gray Dolomitic Boulder	11			V	I	S	U	A	L				
841.0	40	50*			Gray Silty Gravel	12	68	6	4	13	94	NP	NP	8				
836.0	46				TOP OF ROCK													
834.0	48		5.0	0.0	Dolomite, gray, very argillaceous, hard. No core loss.													
826.0	54		5.0	0.0	BOTTOM OF BORING													

* Refusal
 Artesian condition encountered at elevation 846.0'.

LOG OF BORING

Date Started 12-10-64 Sampler Type SS Dia. 1 3/8" Water Elev. _____
 Date Completed 12-15-64 Casing Length 45' Dia. 3 1/2" Surface Elev. 882.2'
 Boring No. B-13 Station & Offset 107+16, 65' L/L (FORWARD ABUTMENT)

Elev.	Depth	Std. Pen. (N)	Rec. ft.	Loss ft.	Description	Sample no.	Physical Characteristics							SHTL Class.				
							% Agg.	% C.S.	% F.S.	% Silt	% Clay	LL	PI		W.C.			
882.2	0																	
877.2	5	2/4			Brown and Gray Gravelly Silt and Clay, Trace of Organic	1	42	3	11	23	21	32	11	26				
874.7	8	4/8			Gray Gravelly Clayey Silt	2	34	2	5	24	35	31	10	17				
872.2	10	5/5			Gray Sandy Gravelly Silt	3	20	2	17	40	21	NP	NP	23				
869.7	12	4/12			Gray Gravelly Silt, Trace of Roots	4	26	3	5	31	35	27	5	28				
867.2	16	10/11			Gray Sandy Gravelly Silt	5	43	7	8	20	28	29	6	17				
864.7	18	6/12			Gray Gravelly Silt	6	30	3	6	30	31	26	8	16				
862.2	20	11/15			Gray Gravelly Sandy Silt	7	VI	S	U	A	L	24	7	17				
859.7	22				Gray Sand (Wash Sample)	8			V	I	S	U	A	L				
857.2	26	9/14			Gray Gravelly Silt	9	27	8	6	30	29	24	7	14				
852.2	30	21/24			Gray Silty Sandy Gravel	10	60	12	10	11	7	NP	NP	14				
847.2	36				Brown Sand (Wash Sample)	11			V	I	S	U	A	L				
842.2	40	50/6			Light Brown Silty Sandy Gravel	12	49	7	8	29	7	NP	NP	13				
837.2	46				TOP OF ROCK													
835.0	48		5.0	0.0	Dolomite, gray, extremely argillaceous, hard. No core loss.													
827.2	54		5.0	0.0	BOTTOM OF BORING													

* Refusal

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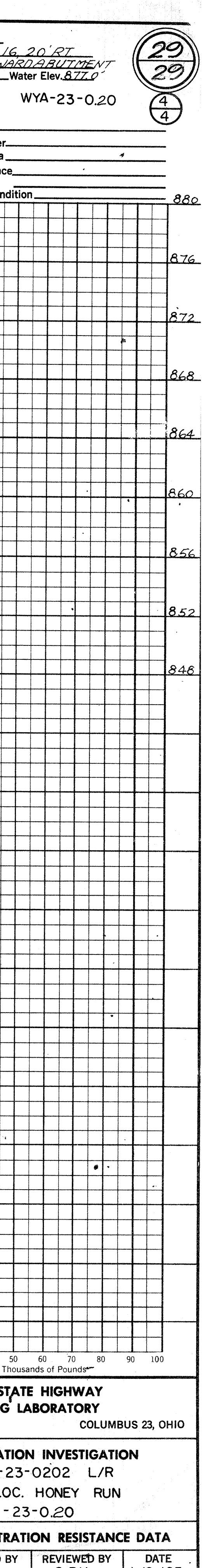
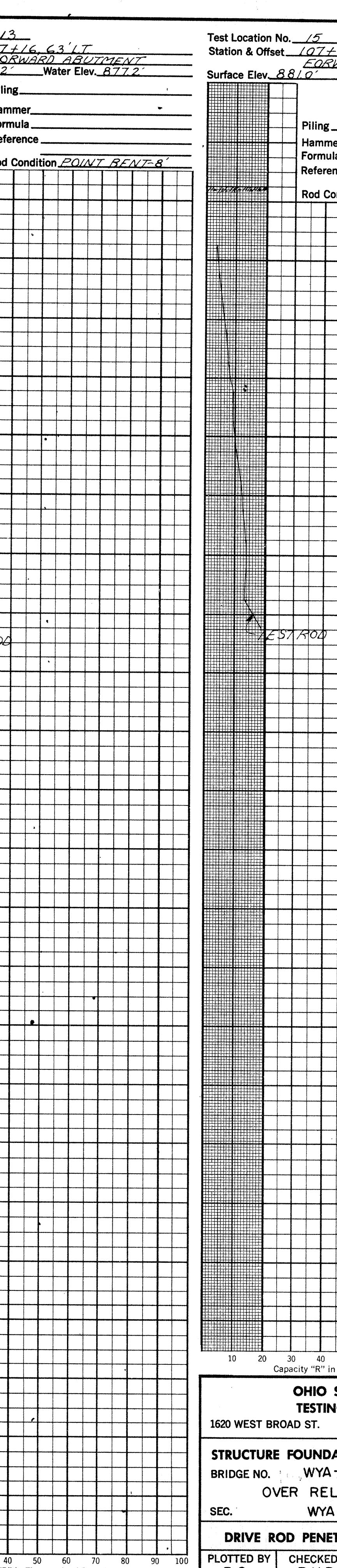
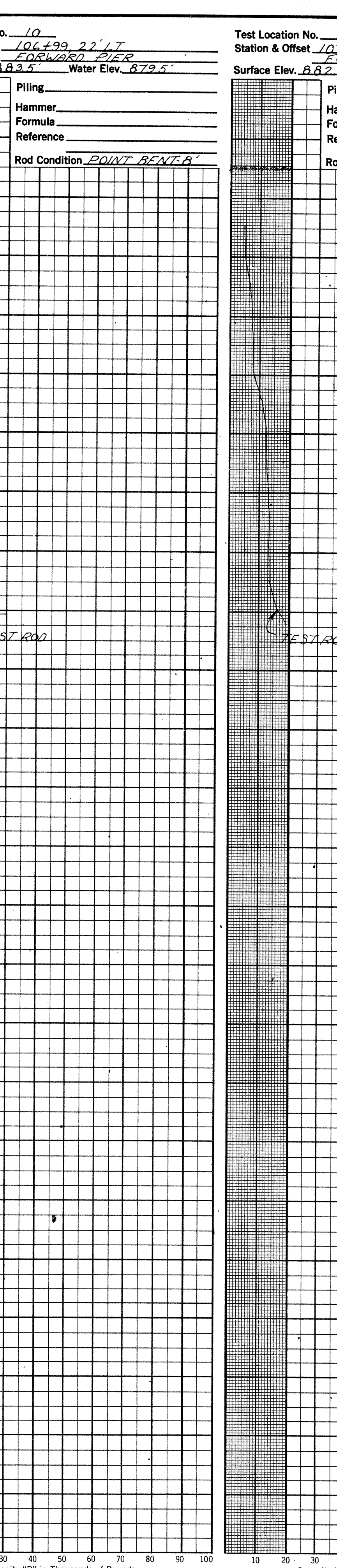
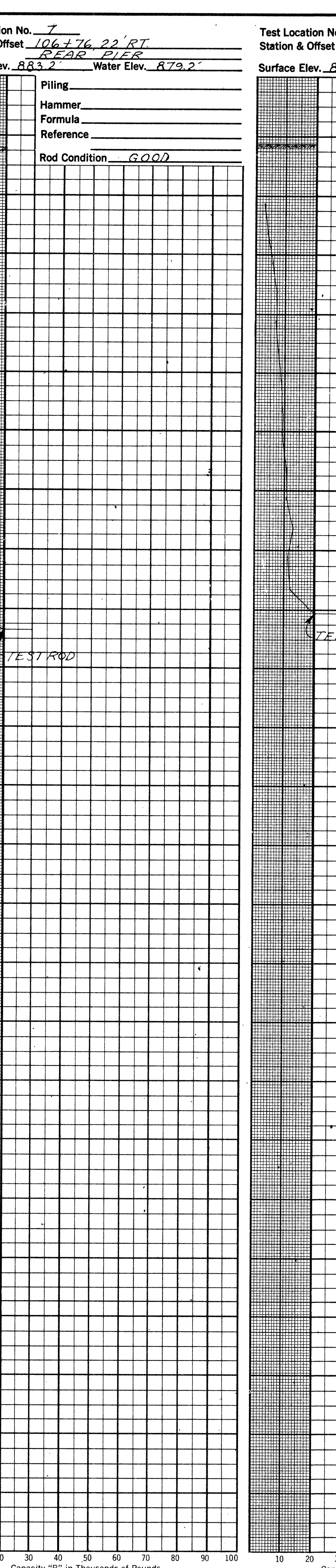
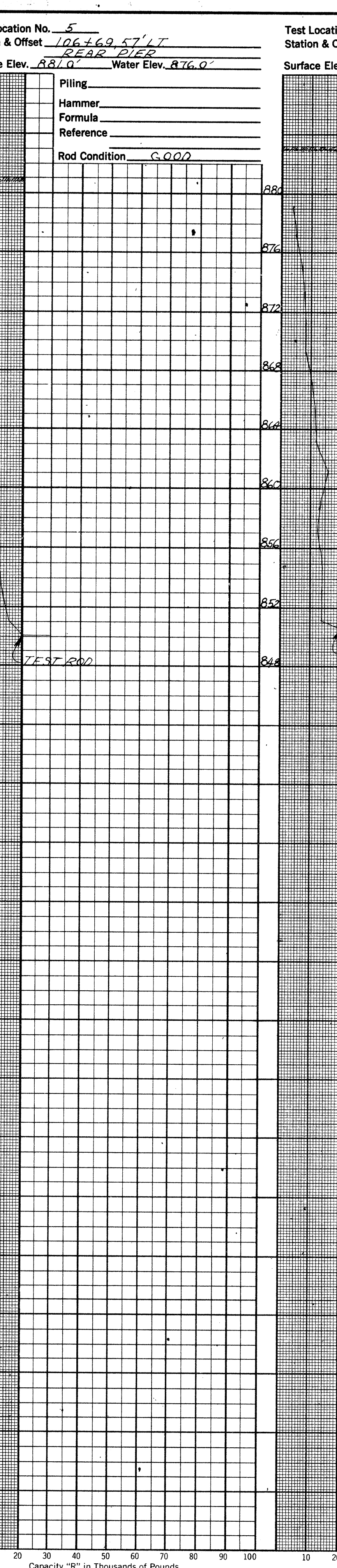
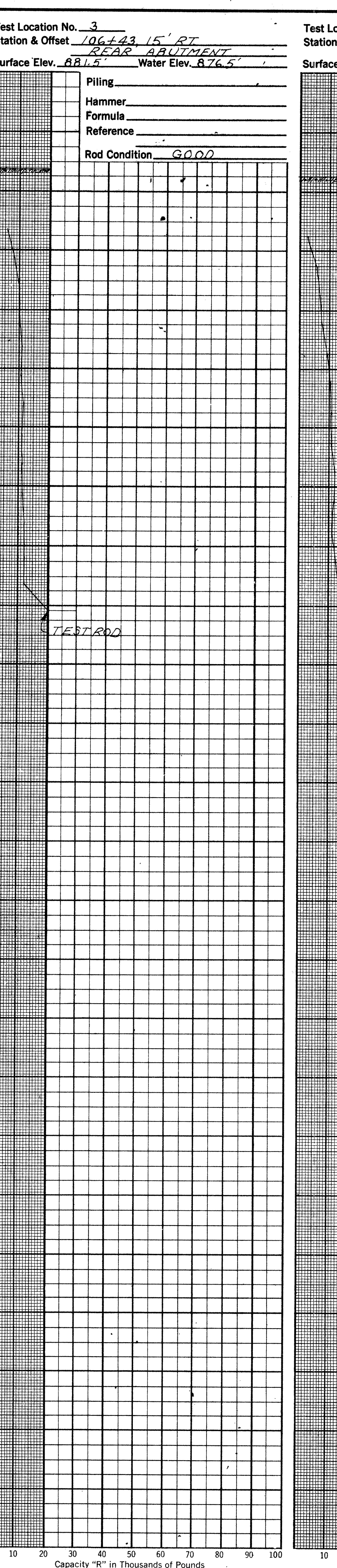
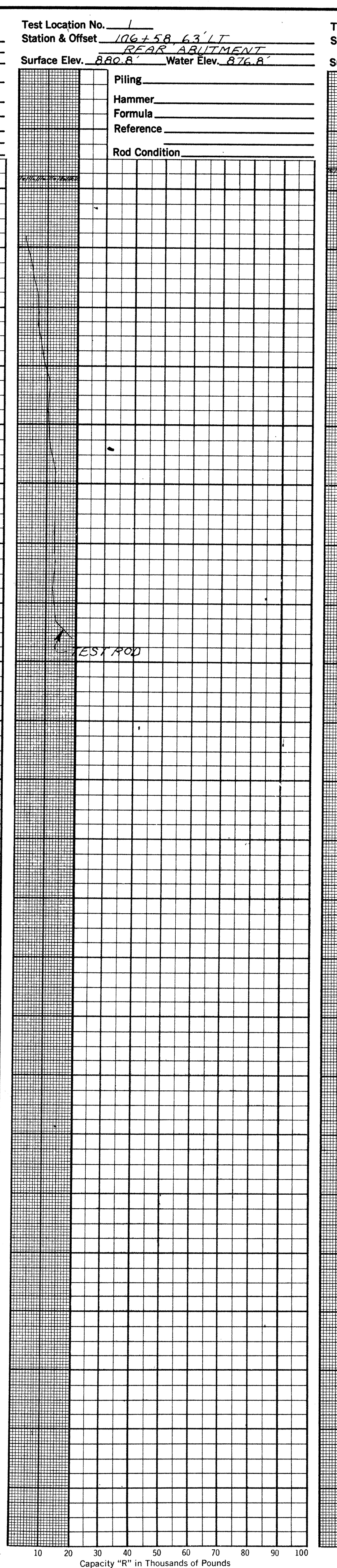
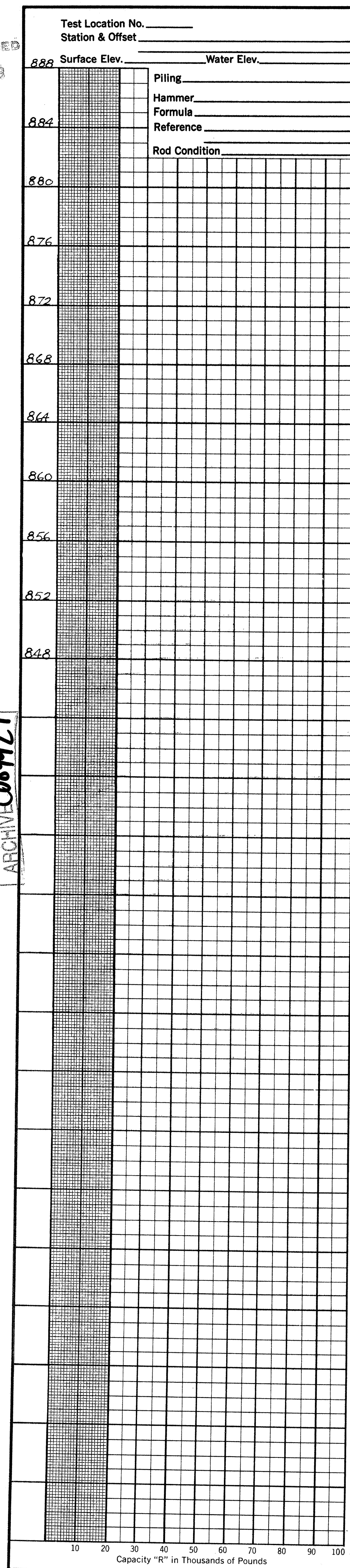
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 BRIDGE NO. WYA-23-0202 L/R
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BORING DATA

TYPED BY P.A.S.	CHECKED BY R.C.	REVIEWED BY G.P.H.	DATE 1/6/65
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DRIVE ROD PENETRATION RESISTANCE DATA

PLOTTED BY R.C.	CHECKED BY R.H.P.	REVIEWED BY G.P.H.	DATE 1/6/65
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