



# **UNDERWATER BRIDGE**

# **INSPECTION REPORT**

STRUCTURE NO. 4800451 (LUC-2-3175) SR-2 OVER CEDAR CREEK LUCAS COUNTY, OH DISTRICT 2

April 2020

Prepared for:



JOSHUA M. JOHNSON E-76141

10/9/2020

Prepared by:



124 Venture Court, Suite 10 Lexington, Kentucky 40511 859.367.0097 • www.collinsengr.com



## TABLE OF CONTENTS

EXEC	CUTIVE SUMM	ARY						
1.0	INTRODUCT	INTRODUCTION						
	1.1 Purpo	se and Scope1						
	1.2 Gener	al Description of the Structure1						
	1.3 Metho	d of Investigation1						
2.0	EXISTING CO	EXISTING CONDITIONS						
	2.1 Gener	al Conditions2						
	2.2 Substr	Substructure Conditions						
	2.2.1	Abutment 1						
	2.2.2	Bent 1						
	2.2.3	Bent 2						
	2.2.4	Abutment 2						
3.0	EVALUATIO	N AND RECOMMENDATIONS4						
EXHI	BIT 1 – FIGURI	ES						
	LOCATION N	1AP						
	SOUNDING I	PLAN						
	CHANNEL C	ROSS SECTIONS - UPSTREAM						
CHANNEL CROSS SECTIONS - CENTERLINE								
	CHANNEL C	ROSS SECTIONS - DOWNSTREAM						
	ABUTMENT	1						
	BENT 1							
	BENT 2							
	ABUTMENT	2						
EXHI	BIT 2 – INSPEC	TION PHOTOGRAPHS15						
	Photograph No	D. 1: Overall View of Structure No. 4800451 (LUC-2-3175), Looking South.						
	Photograph No	D. 2: Overall View of Structure No. 4800451 (LUC-2-3175), Looking North.						
	Photograph No	b. 3: View of the West Embankment Upstream of the Structure, Looking West.						
	Photograph No	b. 4: View of the West Embankment at the Structure, Looking Southwest.						
	Photograph No	b. 5: View of the West Embankment Downstream of the Structure, Looking						
		Northwest.						
	Photograph No	b. 6: View of the East Embankment Upstream of the Structure, Looking Southeast.						
	Photograph No	b. 7: View of the East Embankment at the Structure, Looking Northeast.						
	Photograph No	b. 8: View of the East Embankment Downstream of the Structure, Looking Southeast.						





Photograph No. 9:	View of the East Face of Abutment 1, Looking Northwest.
Photograph No. 10:	View of the East Face of Abutment 1, Looking Southwest.
Photograph No. 11:	View of Spalling on the South End of Abutment 1, Looking Northwest.
Photograph No. 12:	View of the West Face of Bent 1, Looking Northeast.
Photograph No. 13:	View of the East Face of Bent 1, Looking Southwest.
Photograph No. 14:	View of the West Face of Bent 2, Looking Northeast.
Photograph No. 15:	View of the East Face of Bent 2, Looking Southwest.
Photograph No. 16:	View of the West Face of Abutment 2, Looking Southeast.
Photograph No. 17:	View of the South Face of Abutment 2, Looking Northeast.
Photograph No. 18:	View of the Typical Concrete Condition at the Waterline, Looking West.





#### EXECUTIVE SUMMARY

Project:	ODOT District 2 Underwater Bridge Inspections - 2020				
Purpose of Project:	To perform a detailed visual and tactile underwater investigation of underwater bridges for District 2 of the Ohio Department of Transportation.				
Inspection Team:	Team Leader – Joshua Johnson, P.E. – Collins Engineers, Inc. Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc. Team Member – Nicholas Lane – Collins Engineers, Inc.				
Inspection Date(s):	April 21, 2020				
Water Visibility:	0 ft	Water Velocity:	0 ft/s		
Water Temperature:	48 °F	Weather:	Overcast – 45 °F		
Waterline Elevation:	96.2 ft	Type of Boat:	N/A		
Coordinates:	41.638781°N, -83.288514°W				
Access Location:	Gravel Drive at the Southeast	Corner of Structure			
Dive Mode:	Surface Supplied Air				
Waterline Reference:	3.8 ft below the top of deck at the upstream nose of Bent 1.				
Maximum Depth at SS	$U:  7.1  ext{ ft} -  ext{Center of Bent 2}$				
Shoreline Conditions:	The east and west si moderate slopes with		well-vegetated, well-protected,		

#### Summary of Findings:

- Abutment 1:
  - The channel bottom material consisted of loose stone and gravel up to 6 in. diameter.
  - Delamination was observed on 100 percent of the abutment faces.
- Bent 1:
  - The channel bottom material consisted of timber debris of sticks and twigs with silt overlay with approximately 6 in. probe rod penetration.
  - The concrete piles were typically sound and smooth with no defects observed.
  - The concrete pile cap was typically sound and smooth with no defects observed.
  - Heavy timber debris accumulation consisting of logs up to 12 in. diameter were observed along the east face and upstream nose extending from channel bottom to 1 ft above the waterline.
- Bent 2:
  - The channel bottom material consisted of timber debris of sticks and twigs with silt overlay with approximately 6 in. probe rod penetration.
  - $\circ$  The concrete piles were typically sound and smooth with no defects observed.
  - The concrete pile cap was typically sound and smooth with no defects observed.
  - Heavy timber debris accumulation consisting of logs up to 12 in. diameter were observed along the east face and upstream nose extending from channel bottom to 1 ft above the waterline.





### • Abutment 2:

- The channel bottom material consisted of loose stone and gravel up to 6 in. diameter.
- $\circ$  Delamination was observed on 100 percent of the abutment faces.

#### Summary of Recommendations:

- Perform concrete repairs on abutments.
- Remove timber debris from bents.





#### **Underwater Inspection Coding:**

#### **NBI Ratings:**

Item	Description	Coding	Condition
60	Substructure	5 – Fair Condition	Delaminated Concrete
61	Channel	6 - Satisfactory Condition	Timber Debris Accumulation
62	Culvert	N/A	
92B	UW Insp. Frequency	60 Months	
93B	Insp. Date	04 21 20	
113	Scour Critical Bridges	5 – Within Foundation Limits	Stable (Inspector Recommended)

#### **AASHTO National Bridge Element (NBE) Ratings:**

					Conditi	on State	
Element #	Description	Units	Total	1	2	3	4
215	Reinforced Concrete Abutment	LF	120	0	0	120	0
226	Prestressed Concrete Pile	EA	18	18	0	0	0
234	Pier Cap, Reinforced Concrete	LF	100	100	0	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.





#### 1.0 INTRODUCTION

#### 1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the SR-2 Bridge over Maumee River in Lucas County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for District 2 of the Ohio Department of Transportation (ODOT) on April 21, 2020. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Obtain channel profile cross sections at the upstream and downstream fascias.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

#### 1.2 <u>General Description of the Structure</u>

Structure No. 4800451 spans 95 ft, carrying SR-2 over Cedar Creek and is approximately 40 ft wide. The bridge superstructure is constructed of four precast concrete slab spans. The roadway orientation of the longitudinal axis of the bridge is west to east. The substructure units are labeled as Abutments 1 and 2 and Bents 1 and 2. Existing design drawings were not available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

#### 1.3 <u>Method of Investigation</u>

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.





A three-person team consisting of a professional engineer-diver and team leader (Joshua Johnson, P.E.), an engineer-diver (Matthew Rogers, E.I.T.), and a technician-diver (Nicholas Lane) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from the shore and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater Photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod, digital fathometer, and pneumofathometer. Soundings were collected at quarter points along the bridge centerline as well as at quarter points along the upstream and downstream fascias and 50 ft fascias. Additional soundings were collected adjacent to Bents 1 and 2 and at 10 feet intervals in-line with the bents, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

#### 2.0 EXISTING CONDITIONS

#### 2.1 <u>General Conditions</u>

At the time of the inspection, the waterline of 4800451 (LUC-2-3175) was located approximately 3.8 ft below the top of deck at the upstream nose of Bent 1, which corresponds to a waterline elevation of 96.2 ft. During the inspection, the waterway was flowing at approximately 0 ft per second. The bridge bent skew was consistent with the channel alignment and does not require attention at this time. The east and west shorelines consisted of well-vegetated, well-protected, moderate slopes with no erosion. Refer to Photographs 3 through 8 in Exhibit 2 for views of the shorelines near the structure.





#### 2.2 <u>Substructure Conditions</u>

#### 2.2.1 Abutment 1

The channel bottom material consisted of loose stone and gravel up to 6 in. diameter. Delamination was observed on 100 percent of the abutment faces. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Abutment 1. Refer to Photographs 9 through 11 in Exhibit 2 for views of Abutment 1.

#### 2.2.2 Bent 1

The channel bottom material consisted of timber debris of sticks and twigs with silt overlay with approximately 6 in. probe rod penetration. The concrete piles were typically sound and smooth with no defects observed. The concrete pile cap was typically sound and smooth with no defects observed. The concrete pile cap was typically sound and smooth with no defects observed. Heavy timber debris accumulation consisting of logs up to 12 in. diameter were observed along the east face and upstream nose extending from channel bottom to 1ft above the waterline. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Bent 1. Refer to Photographs 12 and 13 in Exhibit 2 for views of Bent 1.

#### 2.2.3 Bent 2

The channel bottom material consisted of timber debris of sticks and twigs with silt overlay with approximately 6 in. probe rod penetration. The concrete piles were typically sound and smooth with no defects observed. The concrete pile cap was typically sound and smooth with no defects observed. The concrete pile cap was typically sound and smooth with no defects observed. Heavy timber debris accumulation consisting of logs up to 12 in. diameter were observed along the east face and upstream nose extending from channel bottom to 1ft above the waterline. Refer to Figure 8 in Exhibit 1 for detailed inspection notes of Bent 2. Refer to Photographs 14 and 15 in Exhibit 2 for views of Bent 2.

#### 2.2.4 Abutment 2

The channel bottom material consisted of loose stone and gravel up to 6 in. diameter. Delamination was observed on 100 percent of the abutment faces. Refer to Figure 9 in Exhibit 1 for detailed inspection notes of Abutment 2. Refer to Photographs 16 through 18 in Exhibit 2 for views of Abutment 2 and typical concrete condition at the waterline.





#### 3.0 EVALUATION AND RECOMMENDATIONS

Overall, the inspected substructure units of Structure No. 4800451 (LUC-2-3175) were in fair condition. The spalls and associated concrete delaminations at Abutments 1 and 2 are not structural concerns at this time; however, they should be repaired to prevent further deterioration. The repairs should include removal of unsound concrete to a minimum of 1 inch behind the reinforcing steel, cleaning and replacing reinforcing steel as required, and placing concrete designed to provide high durability with low permeability.

The timber debris accumulations at Bents 1 and 2 is obstructing channel flow and should be removed at this time. Removal of the timber debris will reduce excessive lateral loads on the bent, limit further debris accumulation, and reduce the likelihood of channel bottom degradation resulting from obstructed flow.

It is recommended that the submerged substructure units of Structure No. 4800451 (LUC-2-3175) be next inspected underwater at an interval not to exceed 60 months, no later than April 21, 2025.

Respectfully Submitted, COLLINS ENGINEERS, INC.

Lh (

Joshua Johnson, P.E. Project Manager

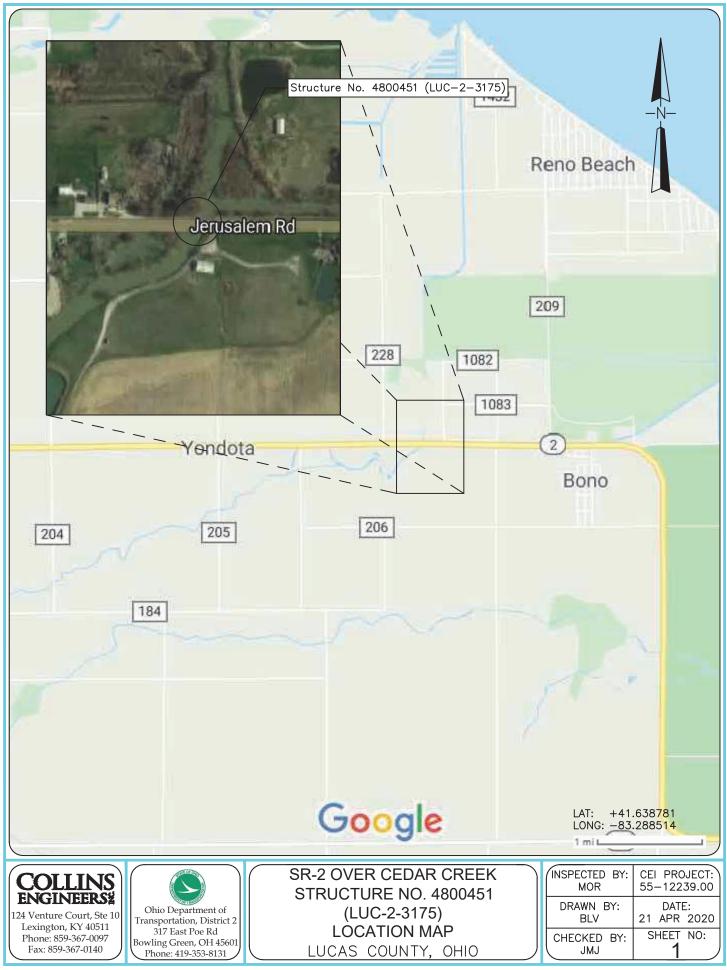
Originated by: Kevin Mitchell, E.I.T.



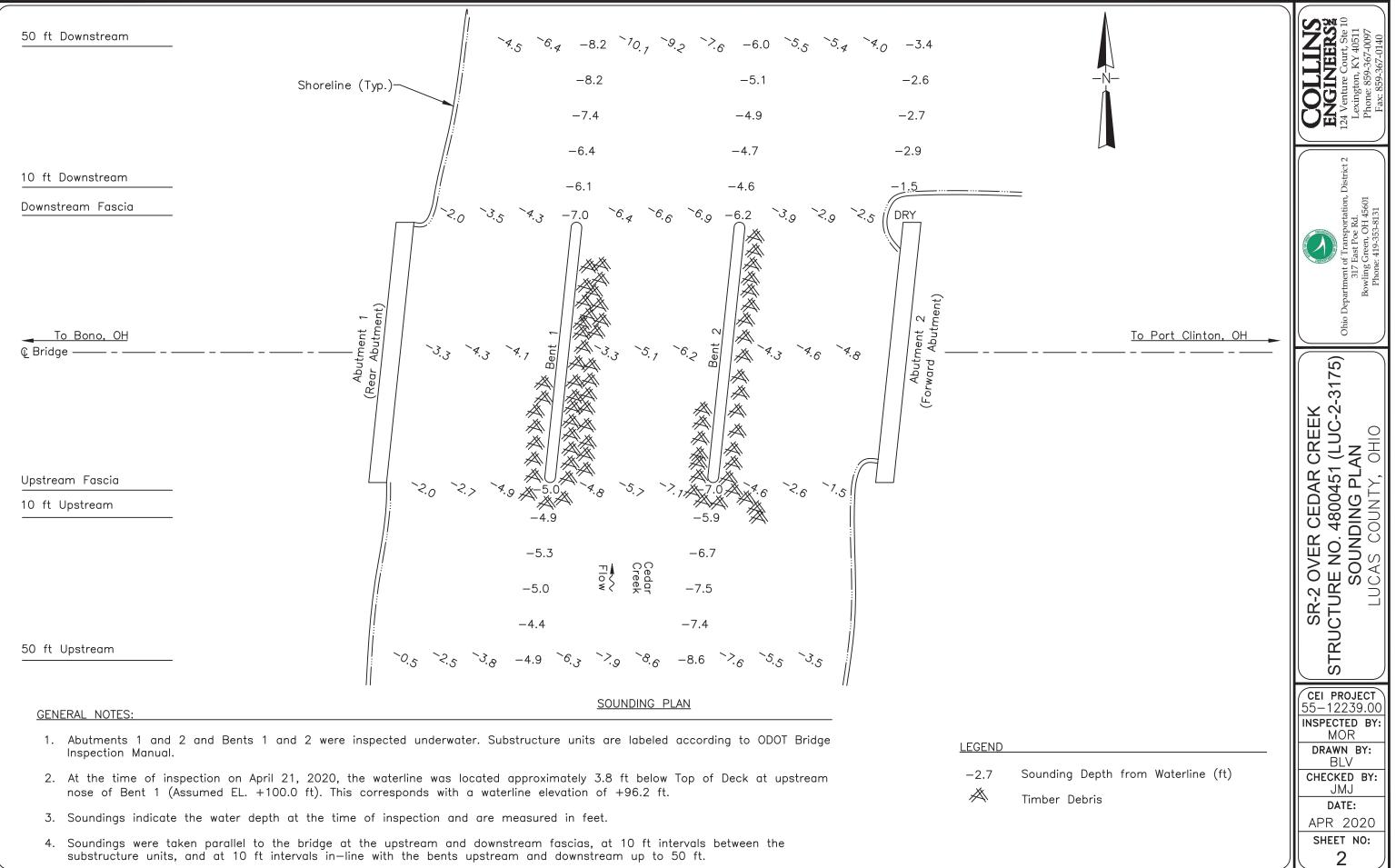


# EXHIBIT 1 – FIGURES

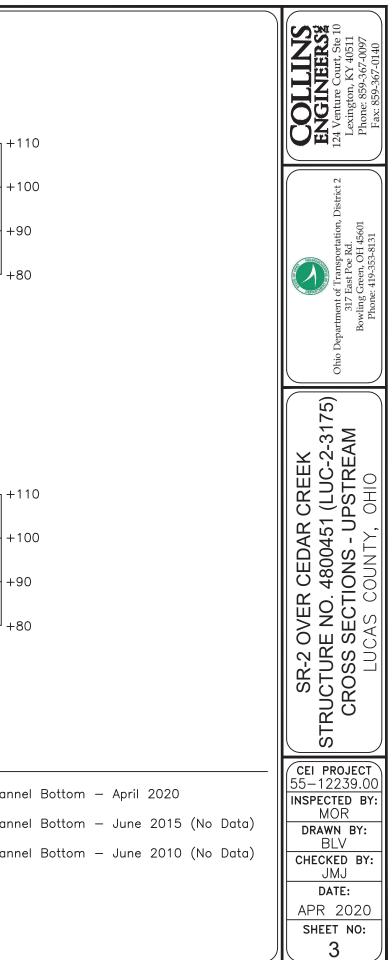


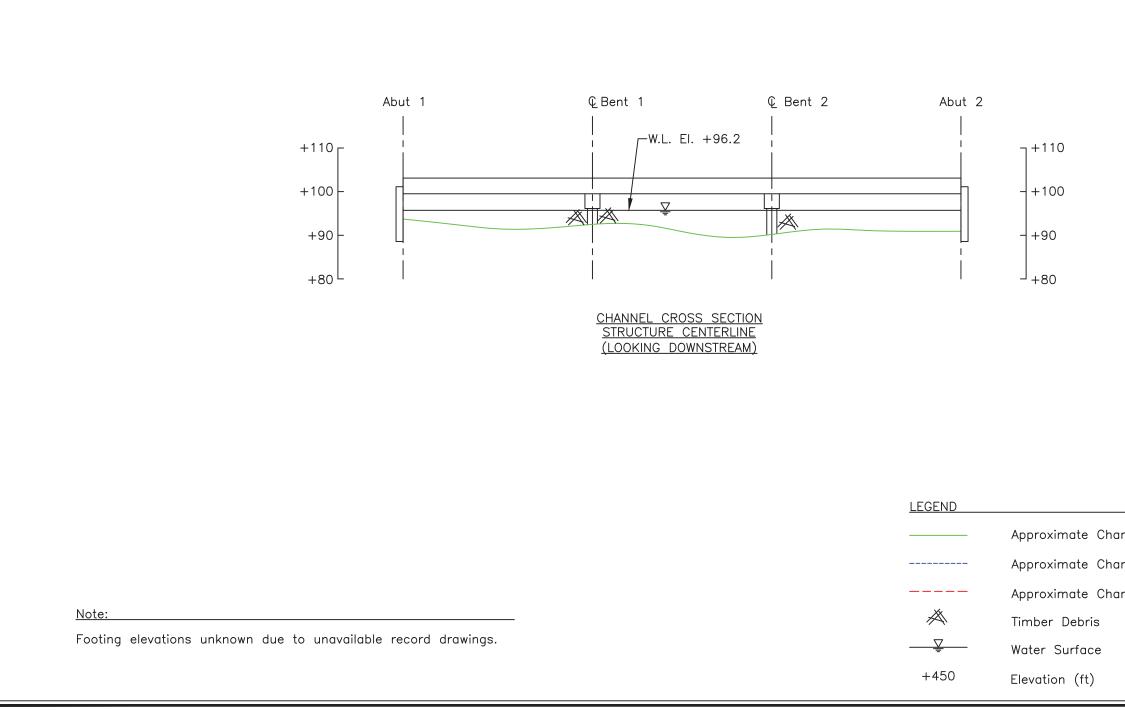


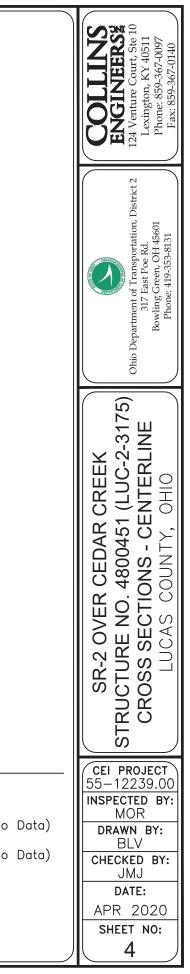
Y:\PROJECTS\55 - LEXINGTON\55-12239.00 - ODOT D2 UW INSP 2020\ENGINEERING\BRIDGES\4800451\4800451 (LUC-2-3175).DWG9/25/2020 5:13:55 PM



ING DOV	I FASCIA WNSTREAM		Abut 1	€ Bent 1	€ Bent 2	Abut 2
ation	Y(ft)*			 ┌─₩.L. EI. +96.2	,   ,	
A1 /4	<u>3.8</u> 5.8	+110			-	_ 
/2	6.5					
/4	8.7	+100 -				
31	8.3					
/4	8.6	+90 -			24	
/2	9.5					
/4 2	10.5	+80 <sup>L</sup>	I	I	I	· -
<u>/</u> 4	8.4			<u>CHANNEL CROSS SE</u> <u>UPSTREAM FASC</u>	ECTION NA	
2	6.4			(LOOKING DOWNSTE		
′4	5.3					
2	2.0					
file tak p of d	ken from eck to bottom					
nannel	bottom					
			Abut 1	€ Bent 1	© Bent 2	Abut 2
		+110 -		l ┌─₩.L. EI. +96.2	2	
		+100-				
				∳ <sub>₹</sub>		
		+90 -				
		+80				.
		100		CHANNEL CROSS SE	ECTION	
				<u>50 FT UPSTREA</u>	<u>\M</u>	
				<u>(LOOKING DOWNSTE</u>	<u>KLAM)</u>	
					LEGEND	
						Approximate Ch
						Approximate Ch
						Approximate Ch
<u>Note</u>	2:				Å	
		due to unavailable record dr	rawings.			Timber Debris
1 000	ang diatadions unknown		amingo.			Water Surface



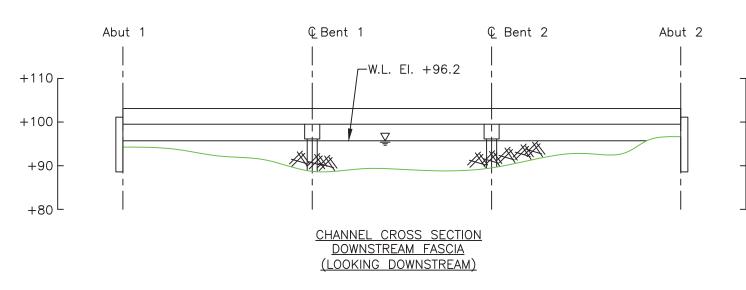


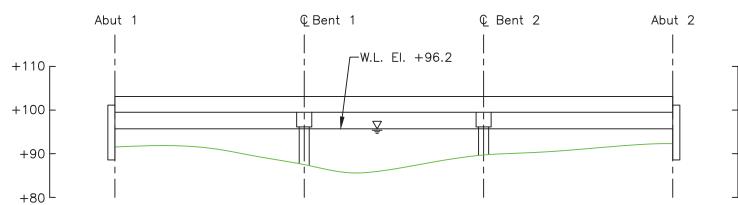


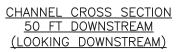
Approximate Channel Bottom - April 2020 Approximate Channel Bottom - June 2015 (No Data) Approximate Channel Bottom - June 2010 (No Data)

DOWNSTREAM FASCIA LOOKING DOWNSTREAM				
Location	Y(ft)*			
A1	3.8			
1/4	5.8			
1/2	7.3			
3/4	8.1			
B1	9.9			
1/4	10.5			
1/2	10.4			
3/4	10.7			
B2	8.9			
1/4	7.7			
1/2	7.7			
3/4	6.3			
A2	3.8			
*	1 (			

\*Profile taken from top of deck to channel bottom

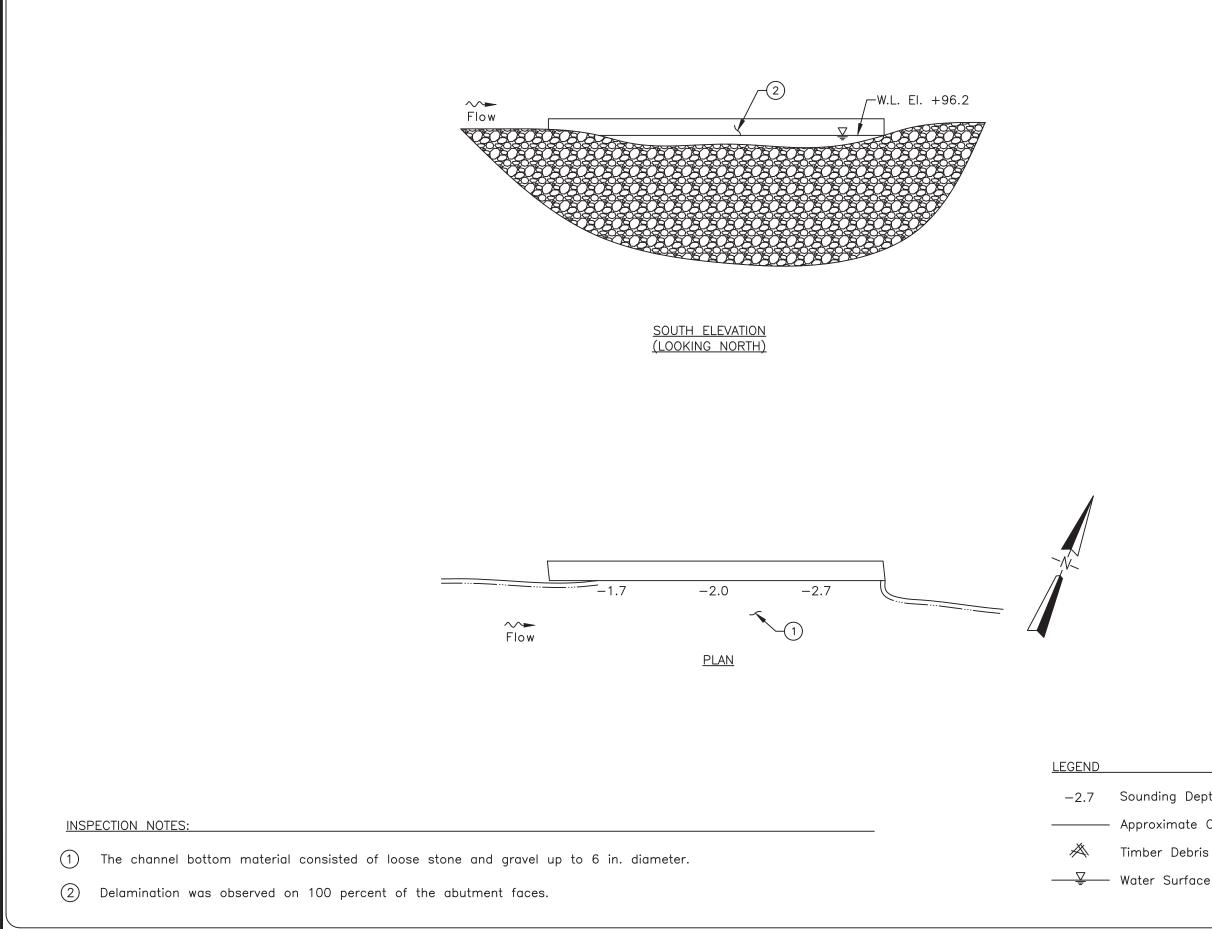


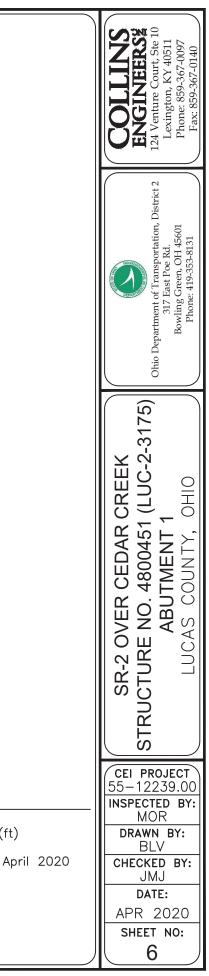




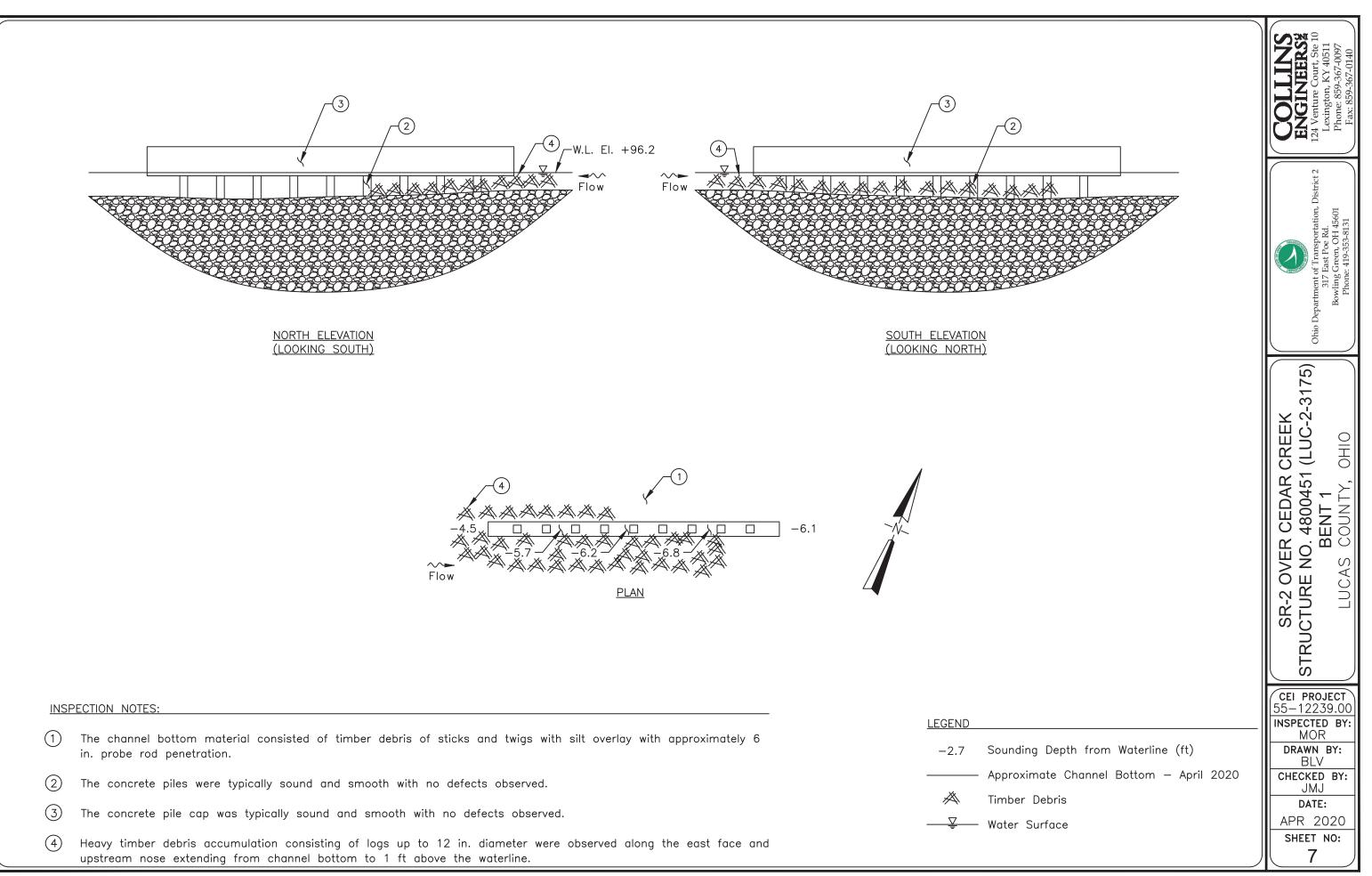


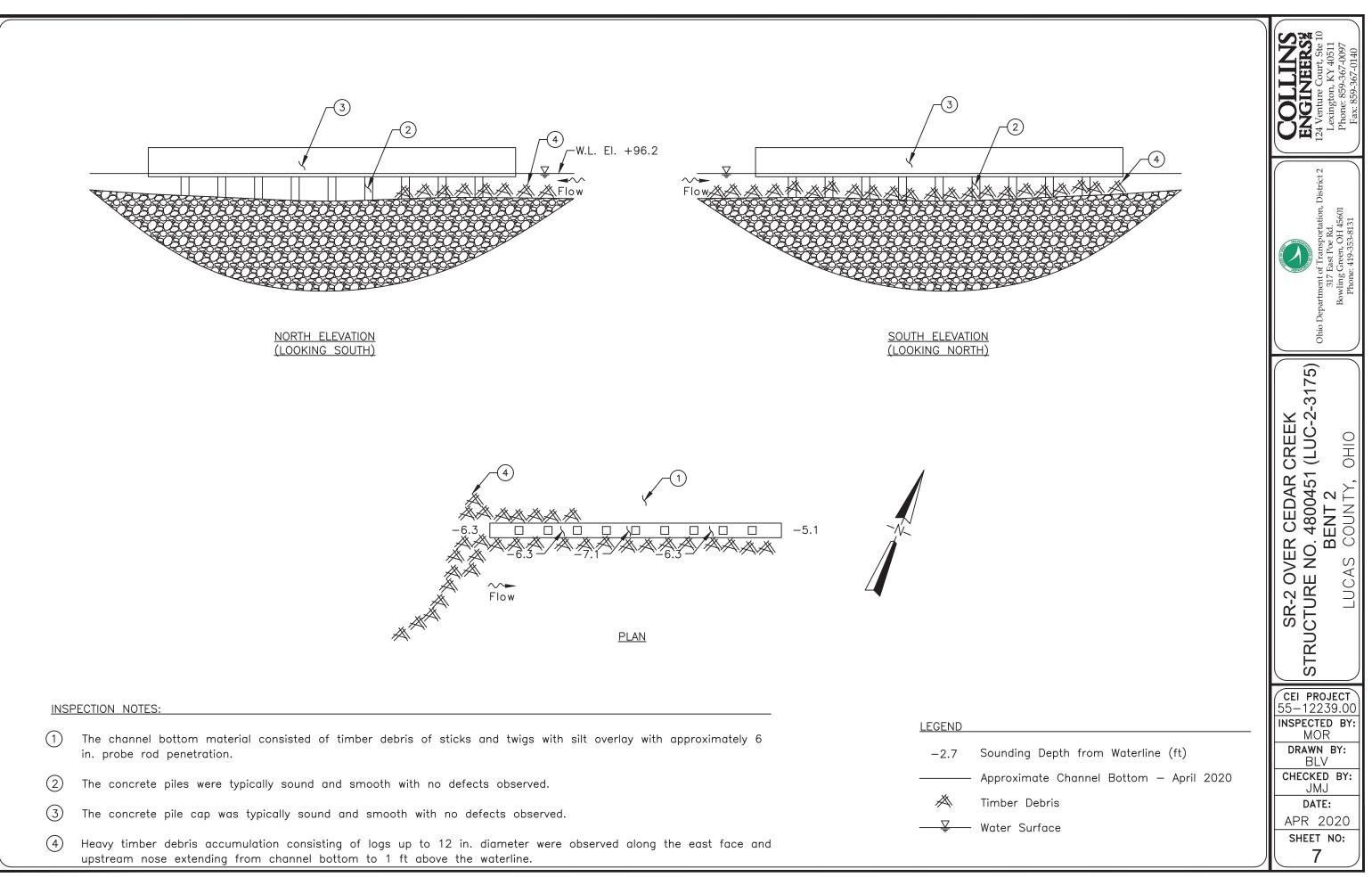
+110	<b>COLLINS</b> <b>ENGINEERS</b> 124 Venture Court. Ste 10 Lexington, KY 40511 Phone: 859-367-0097 Fax: 859-367-0140
+100	strict 2
+90	:tation, Di d. [131
+80	Ohio Department of Transportation, District 2 317 East Poe Rd. Bowling Green, OH 45601 Phone: 419-353-8131
+110 +100 +90 +80	B STRUCTURE NO. 4800451 (LUC-2-3175) CROSS SECTIONS - DOWNSTREAM LUCAS COUNTY, OHIO
annel Bottom — April 2020	55-12239.00 INSPECTED BY: MOR
annel Bottom — June 2015 (No Data)	DRAWN BY: BLV
annel Bottom — June 2010 (No Data)	CHECKED BY: JMJ DATE: APR 2020 SHEET NO: 5

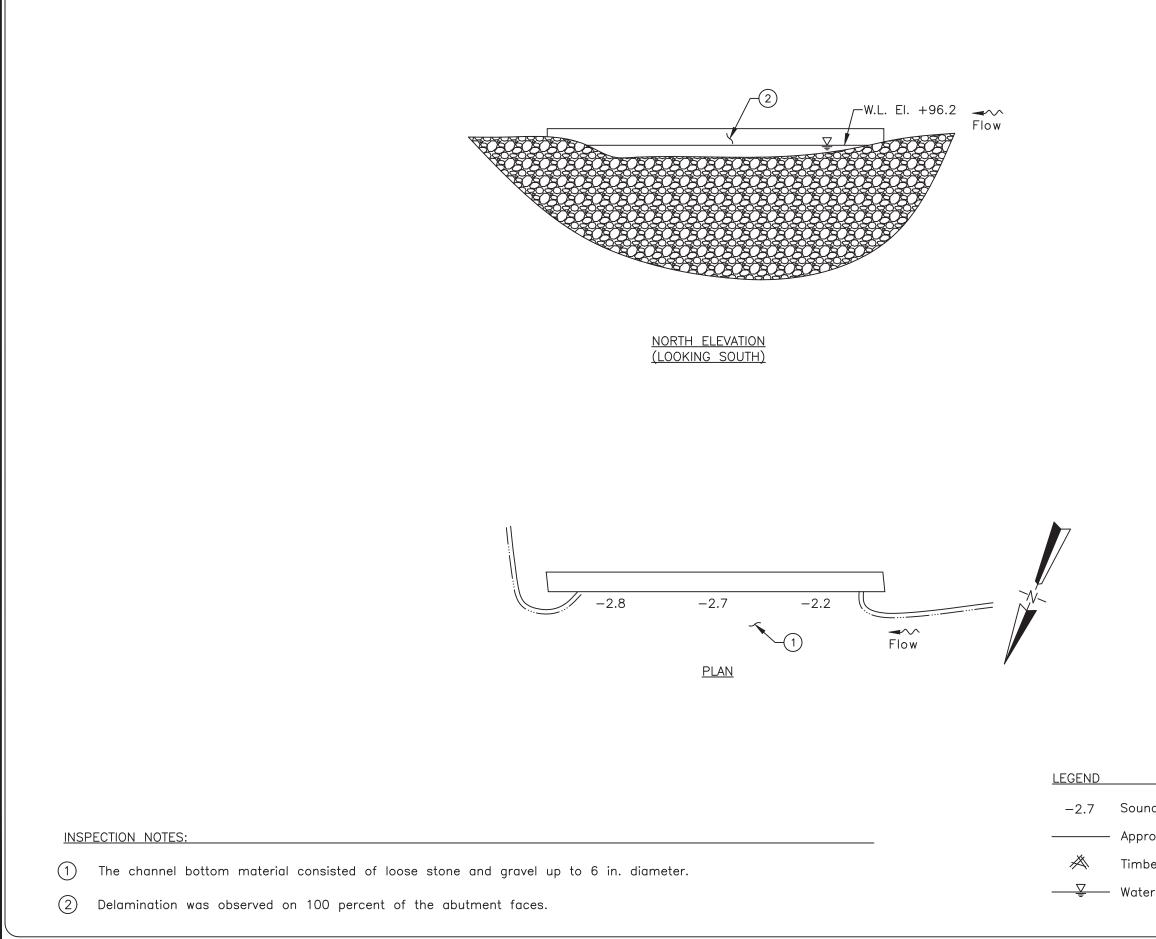




Sounding Depth from Waterline (ft) Approximate Channel Bottom — April 2020 Timber Debris Water Surface









ding	De	pth	from	Waterlir	ne	(ft)	
oximo	ate	Ch	annel	Bottom	_	April	2020
er D	ebri	is					
r Sui	rfac	e					



# EXHIBIT 2 – INSPECTION PHOTOGRAPHS







Photograph No. 1: Overall View of Structure No. 4800451 (LUC-2-3175), Looking South.



Photograph No. 2: Overall View of Structure No. 4800451 (LUC-2-3175), Looking North.







Photograph No. 3: View of the West Embankment Upstream of the Structure, Looking West.



Photograph No. 4:

View of the West Embankment at the Structure, Looking Southwest.







Photograph No. 5: View of the West Embankment Downstream of the Structure, Looking Northwest.



Photograph No. 6:

View of the East Embankment Upstream of the Structure, Looking Southeast.







Photograph No. 7: View of the East Embankment at the Structure, Looking Northeast.



Photograph No. 8: View of the East Embankment Downstream of the Structure, Looking Southeast.







Photograph No. 9: View of the East Face of Abutment 1, Looking Northwest.



Photograph No. 10: View of the East Face of Abutment 1, Looking Southwest.







Photograph No. 11: View of Spalling on the South End of Abutment 1, Looking Northwest.



Photograph No. 12: View of the West Face of Bent 1, Looking Northeast.







Photograph No. 13: View of the East Face of Bent 1, Looking Southwest.



Photograph No. 14:

View of the West Face of Bent 2, Looking Northeast.







Photograph No. 15: View of the East Face of Bent 2, Looking Southwest.



Photograph No. 16: View of the West Face of Abutment 2, Looking Southeast.







Photograph No. 17: View of the South Face of Abutment 2, Looking Northeast.



Photograph No. 18: View of the Typical Concrete Condition at the Waterline, Looking West.





# EXHIBIT 3 – UNDERWATER DIVE INSPECTION PROCEDURE

# CHECKLIST



#### **Underwater Dive Inspection Procedure Checklist**

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a standalone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

#### I. Bridge Identification

a.	Agency with Inspection	Responsibility:	ODOT DISTRICT 2		
	Dive Frequency:	<u>60 months</u>			
	SFN: <u>4800451</u> Bridge	Number	(County-Route-SLM-SD): LUC-2-3175		
Supers	tructure Type	Main Span Typ	e: <u>REINFORCED CONCRETE</u>		
		Approach Span	: <u>REINFORCED CONCRETE</u>		
Substru	ucture Type	Abutment Type: <u>REINFORCED CONCRETE</u>			
		Pier Type:	REINFORCED CONCRETE		
		Total Pier Count: <u>2</u>			
		Total Pier Coun	t in water: <u>2</u>		
		Foundations:	UNKNOWN		
Feature	e Intersected	CEDAR CREEK			

## b. Photographs

### Endview



Elevation



Underside

#### II. Office and Field Assessment

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

a. Channel Conditions b. Anticipated Water conditions which \_\_\_\_Waterway features may affect the inspection Rapid stream flows, \_Cold Water (Apprx. Temp\_\_\_\_) X Significant debris accumulation Black water Rapid stream flows Constricted waterway openings Soft or unstable streambeds Near military facility \_\_\_\_Meandering channels \_\_\_\_Tribal fishing \_Other which may promote scour and \_Water quality undermining of substructure elements \_\_\_\_History of Log jams Navigable Waterway Flow Controls c. Identify factors that may accelerate the deterioration of the bridge elements: Highly corrosive water Unprotected steel members Other

**Risk Factor Narrative:** 

#### III. Contacts Prior to Work

District 2 Bridge Engineer: David Geckle, P.E.

Email: <u>david.geckle@dot.ohio.gov</u> – Phone: 419-373-4377

Point of contact for immediate action such as closing the bridge due to findings

Contact Bridge Owner \_\_\_\_\_14 (number) days before the proposed underwater inspection.

Special contracting and scheduling procedures prior to inspection, include recommended lead time

Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard			
Property Owner			
Access Equipment			
Lake or River draw- down			
Canal dry time			
Tree removal			
Other:			
Other:			

#### IV. Dive Team Shall Include the Following:

#### Dive Team Narrative:

The dive team consisted of one Team Leader (NBIS, P.E., ADCI) and two Team Members (NBIS, UW, ADCI).

*Example: The Bridge shall be investigated using a three-member dive team: one supervisor to monitor rack box and take notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.* 

#### V. <u>Site Information</u>

Navigable waterway:	Y / <u>N</u>	Anticipated currentft
If Yes, waterway river point		Scour Critical (item 113): <u>5</u>
Anticipated water visibility depth	n0ft	POA in place: Y/ <u>N</u>
Anticipated Dive depth	<u>7</u> ft	Scour Monitoring devices present: Y/ <u>N</u>

Verify the Scope of Services when work is contracted for the procedure for underwater elements that

are not in water during an inspection.

Site Information Narrative:

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with		
		Commentary		
Piers and Number of	2	100 % LEVEL I 10% LEVEL II		
Columns	2			
Abutment	2	100 % LEVEL I 10% LEVEL II		
Culvert				
Scour Countermeasures				
Fenders or Dolphins				

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

#### VI. Equipment and Field Logistics

a. The inspection should be conducted

using:

\_\_\_\_Chest waders

\_\_\_\_Hip waders

<u>X</u> Diving equipment

SCUBA (Note that ADCI Consensus Standards require communication systems be employed for both SCUBA and Surface-Supplied (whether air or mixedgas) dive modes)

\_\_\_\_SCUBA with communication

X\_\_Surface Supplied with

- communication
- b. The channel bottom should be sounded

utilizing

<u>X</u> Digital fathometer

- <u>X</u> Telescoping survey rod
- <u>X</u> acoustic imaging
- c. During the inspection, the divers should

work from

\_\_\_\_Shore

<u>X</u>\_\_Boat

\_\_\_\_Either

The note taker should work alongside the dive team.

d. Access to the waterway should be obtained from the shore (north bank, southwest quadrant, driveway 30 yards north etc.) GRAVEL DRIVE AT THE SOUTHEAST CORNER OF THE STRUCTURE The maximum depth of the channel is typically measured\_\_\_\_\_ feet from \_\_\_\_\_ 50FT. DOWNSTREATM BETWEEN PIERS 1 AND <u>2.</u> Reference Datum: 3.8FT. BELOW TOP OF DECK AT THE UPSTREAM NOSE OF PIER 1 Soundings should be dictated by the scope of work. When not detailed in the scope they should be repeated from the previous soundings. If neither exist then they need to be taken in a grid pattern between substructure units 100' upstream and 100' downstream.

## VII. Inspection Procedure History

Created: COLLINS ENGINEERS	Date: <u>09/25/2020</u>
Updated By:	Date:

### VIII. Other Narrative Not Included In Previous Sections