

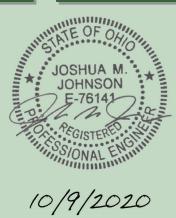
UNDERWATER BRIDGE INSPECTION REPORT

STRUCTURE NO. 6200184 (OTT-2-1312)
SR 2 OVER RUSHA CREEK
OTTAWA COUNTY, OH
DISTRICT 2

April 2020

Prepared for:





Prepared by:

COLLINS ENGINEERS

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Lexington, Kentucky 40511

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EXECUTIVE SUMMARY

Project: ODOT District 2 Underwater Bridge Inspections - 2020

Purpose of Project: To perform a detailed visual and tactile underwater investigation of underwater bridges

for District 2 of the Ohio Department of Transportation.

Inspection Team: Team Leader – Joshua Johnson, P.E. – Collins Engineers, Inc.

Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc.

Team Member – Nicholas Lane – Collins Engineers, Inc.

Inspection Date(s): April 23, 2020

Water Visibility: 0 ft Water Velocity: 0 ft/s

Water Temperature: 45 °F Weather: Overcast – 45 °F

Waterline Elevation: 576.42 ft Type of Boat: Kayak

Coordinates: 41.557128°N, -83.071534°W **Access Location:** Northwest Corner of Structure

Dive Mode: Surface Supplied Air

Waterline Reference: 1.9 ft below the top of pier at the downstream nose of Pier 2.

Maximum Depth at SSU: 8.6 ft – Midpoint on North Face of Pier 2

Shoreline Conditions: The east and west shorelines consisted of well-protected, well-vegetated, mild

slopes with no signs of erosion.

Summary of Findings:

• Abutment 1:

- The channel bottom material consisted of riprap and silt with approximately 6 in. probe rod penetration.
- The submerged portions of the pier were sound and smooth with no defects observed.

• Pier 1:

- The channel bottom material consisted of riprap and silt with approximately 12 in. probe rod penetration.
- o The submerged portions of the pier were sound and smooth with no defects observed.
- O Vertical cracking measuring 1/16 in. was observed throughout the pier faces spaced 6 ft on center.

• Pier 2:

- The channel bottom material consisted of riprap and silt with approximately 12 in. probe rod penetration.
- o The submerged portions of the pier were sound and smooth with no defects observed.
- Vertical cracking measuring 1/16 in. was observed throughout the pier faces spaced 6 ft on center.



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• Abutment 2:

- The channel bottom material consisted of riprap and silt with approximately 6 in. probe rod penetration.
- o The submerged portions of the pier were sound and smooth with no defects observed.
- O Hairline cracking with efflorescence was observed on 30 percent of the abutment face.

Summary of Recommendations:

• Monitor concrete cracking in pier and abutment faces for further propagation.



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Underwater Inspection Coding:

NBI Ratings:

Item	Description	Coding	Condition
60	Substructure	7 – Good Condition	Concrete Cracking
61	Channel	8 – Very Good Condition	No Defects Observed
62	Culvert	N/A	
92B	UW Insp. Frequency	60 Months	
93B	Insp. Date	04 23 20	
113	Scour Critical Bridges	5 – Within Foundation Limits	Stable (Inspector Recommended)

AASHTO National Bridge Element (NBE) Ratings:

					Condition State			
Elem	ent#	Description	Units	Total	1	2	3	4
2	10	Reinforced Concrete Pier Wall	LF	80	80	0	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.



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1.0 INTRODUCTION

1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the SR 2 Bridge over Rusha Creek in Ottawa County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for District 2 of the Ohio Department of Transportation (ODOT) on April 23, 2020. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Obtain channel profile cross sections at the upstream and downstream fascias.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

1.2 General Description of the Structure

Structure No. 6200184 (OTT-2-1312) spans 200 ft, carrying SR 2 over Rusha Creek and is approximately 45.0 ft wide. The bridge superstructure is constructed of three continuous concrete slab spans. The roadway orientation of the longitudinal axis of the bridge is west to east. The substructure units are labeled as Abutments 1 and 2 and Piers 1 and 2. Existing record drawings were available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.



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A three-person team consisting of a professional engineer-diver and team leader (Joshua Johnson, P.E.), an engineer-diver (Matthew Rogers, E.I.T.), and a technician-diver (Nicholas Lane) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from a kayak and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod, and digital fathometer and, and a pneumofathometer. Soundings were collected at quarter points along the bridge centerline as well as at quarter points along the upstream and downstream fascias and 50 ft fascias. Additional soundings were collected adjacent to Piers 1 and 2 and at 10 feet intervals in-line with the piers, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

2.0 EXISTING CONDITIONS

2.1 <u>General Conditions</u>

At the time of the inspection, the waterline of 6200184 (OTT-2-1312) was located approximately 1.9 ft below the top of pier at the downstream nose at Pier 2, which corresponds to a waterline elevation of 576.42 ft. During the inspection, the waterway was flowing at approximately 0 ft per second. The bridge pier skew was consistent with the channel alignment and does not require attention at this time. The east and west shorelines consisted of well-protected, well-vegetated, mild slopes with no erosion. Refer to Photographs 3 through 8 in Exhibit 2 for views of the shorelines near the structure.



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2.2 Substructure Conditions

2.2.1 Abutment 1

The channel bottom material consisted of riprap and silt with approximately 6 in. probe rod penetration. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Abutment 1. Refer to Photographs 4 and 5 in Exhibit 2 for views of Abutment 1.

2.2.2 Pier 1

The channel bottom material consisted of riprap and silt with approximately 12 in. probe rod penetration. The submerged portions of the pier were sound and smooth with no defects observed. Vertical cracking measuring 1/16 in. was observed throughout the pier faces spaced 6 ft on center. Refer to Figure 8 in Exhibit 1 for detailed inspection notes of Pier 1. Refer to Photographs 10 and 11 in Exhibit 2 for views of Pier 1.

2.2.3 Pier 2

The channel bottom material consisted of riprap and silt with approximately 12 in. probe rod penetration. The submerged portions of the pier were sound and smooth with no defects observed. Vertical cracking measuring 1/16 in. was observed throughout the pier faces spaced 6 ft on center. Refer to Figure 9 in Exhibit 1 for detailed inspection notes of Pier 2. Refer to Photographs 12 through 14 in Exhibit 2 for views of Pier 2 and typical concrete condition at the waterline.

2.2.4 Abutment 2

The channel bottom material consisted of riprap and silt with approximately 6 in. probe rod penetration. The submerged portions of the pier were sound and smooth with no defects observed. Hairline cracking with efflorescence observed on 30 percent of the abutment face. Refer to Figure 10 in Exhibit 1 for detailed inspection notes of Abutment 2. Refer to Photographs 6 and 7 in Exhibit 2 for views of Abutment 2.

3.0 EVALUATION AND RECOMMENDATIONS

Overall, the inspected substructure units of Structure No. 6200184 (OTT-2-1312) were in good condition. A comparison of the soundings recorded during the previous inspection on June 25, 2015 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. Although no channel deficiencies were observed, the channel bottom should continue



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to be monitored during future underwater inspections to verify that localized scour or overall channel degradation is not occurring and that the pier footings remain adequately embedded in the channel bottom.

The vertical cracks at Piers 1 and 2 and Abutment 2 are not structural concerns at this time. However, the cracks do provide a means for water infiltration and freeze-thaw damage. It is therefore recommended that these cracks be monitored during future underwater inspections. If further crack propagation or increased crack widths are observed, it may be necessary to repair the cracks by injection or patching with a repair material suitable for underwater applications.

It is recommended that the submerged substructure units of Structure No. 6200184 (OTT-2-1312) be next inspected underwater at an interval not to exceed 60 months, no later than April 23, 2025.

Respectfully Submitted,

COLLINS ENGINEERS, INC.

Joshua Johnson, P.E.

Project Manager

Originated by:

Kevin Mitchell, E.I.T.

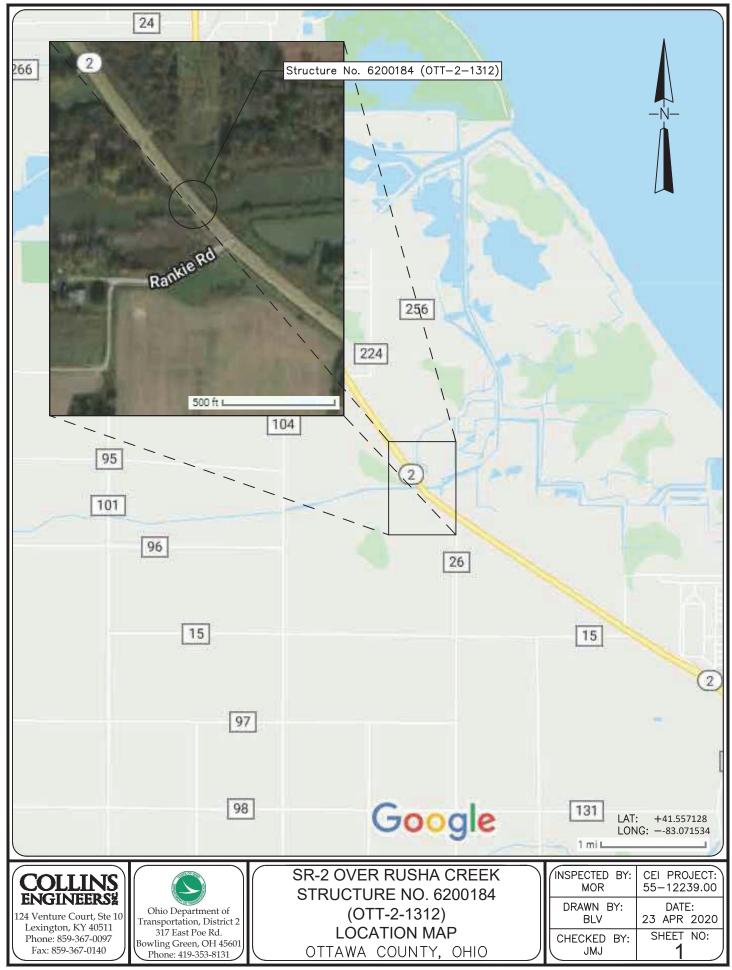


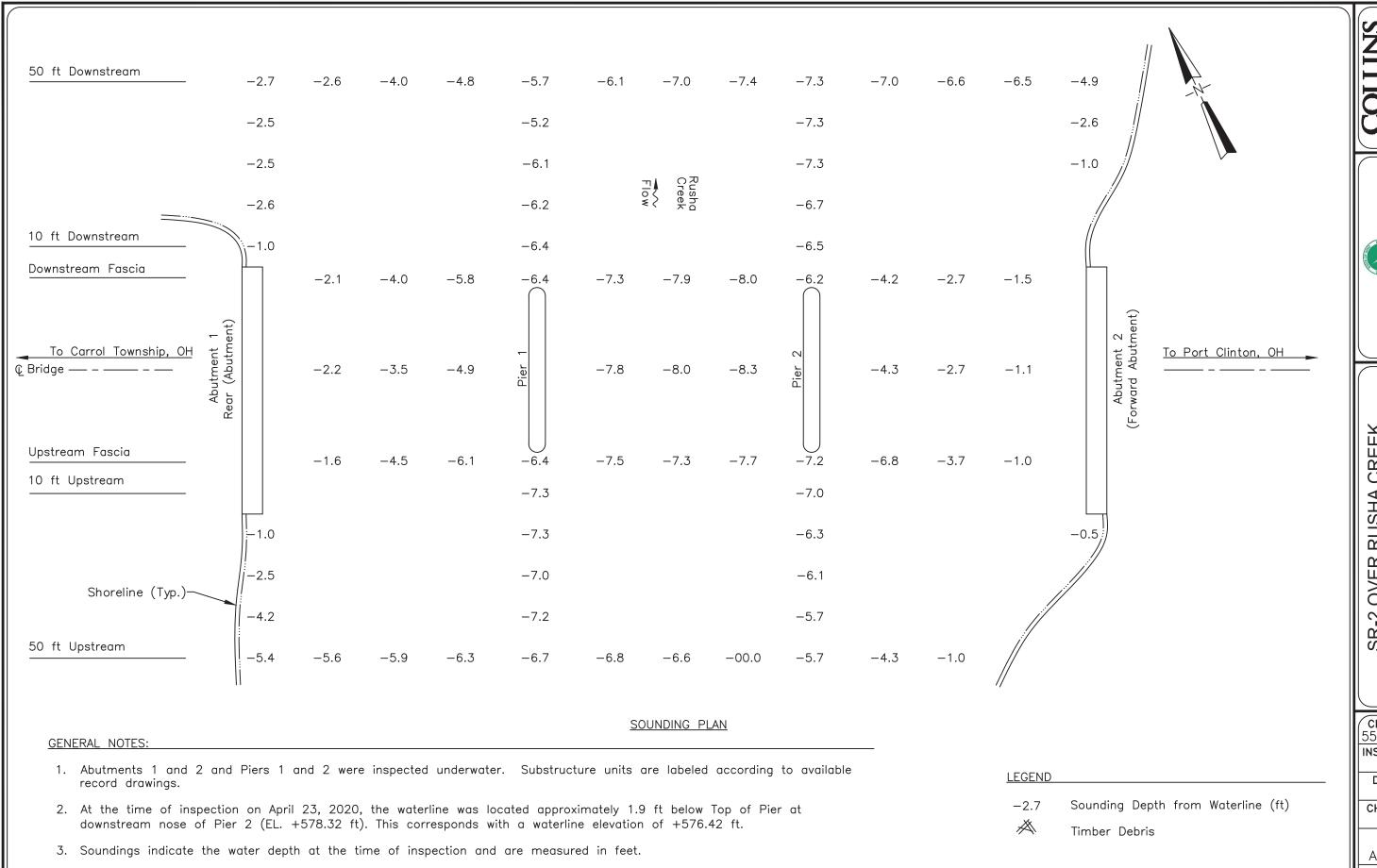
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EXHIBIT 1 – FIGURES







4 (OTT-2-1312) -AN OVER RUSHA C E NO. 6200184 (SOUNDING PLAI SR-2 OVER F STRUCTURE NO. 6 SOUND

CEI PROJECT 55-12239.00 INSPECTED BY: MOR DRAWN BY:

BLV CHECKED BY:

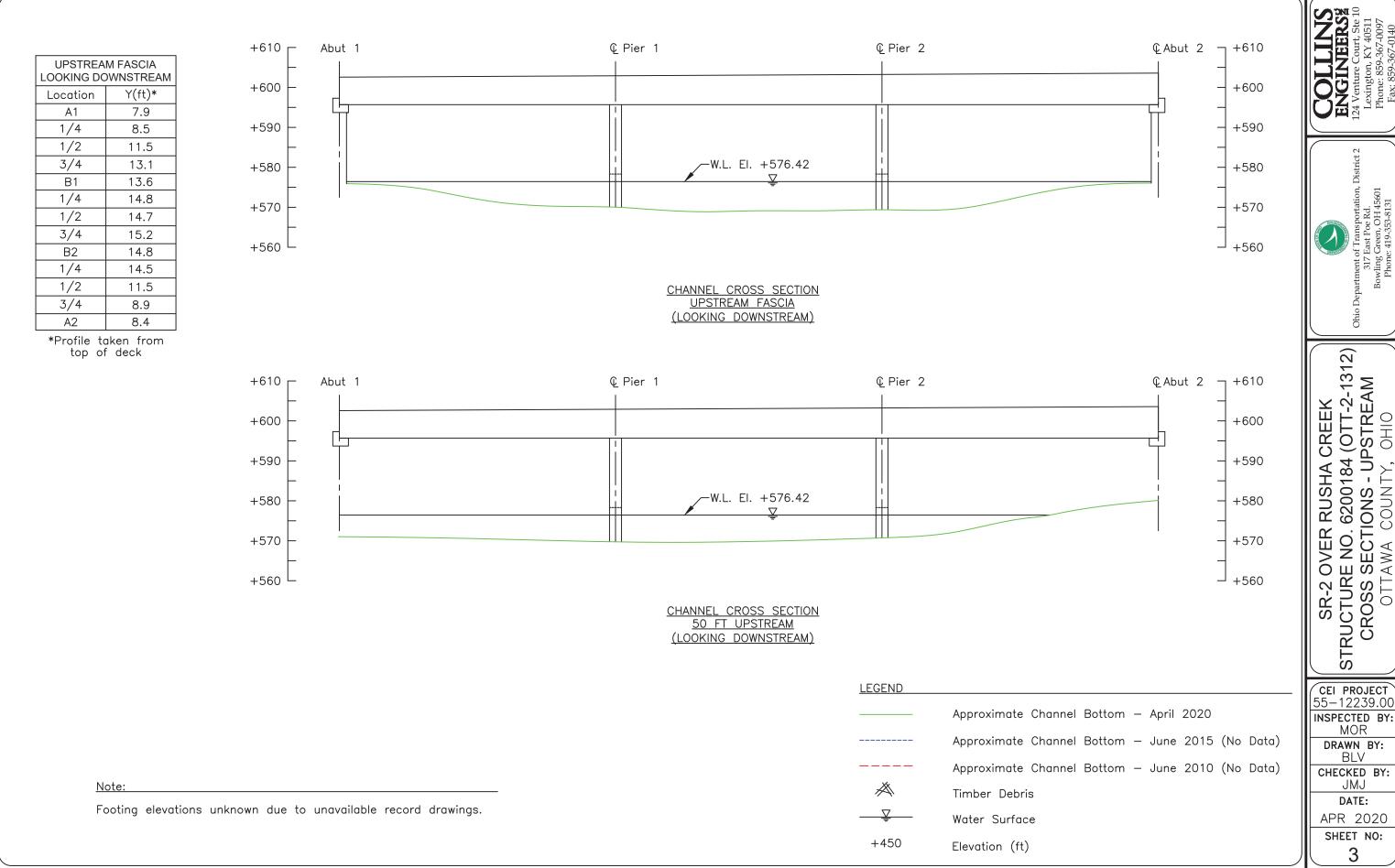
JMJ

DATE:

APR 2020 SHEET NO:

substructure units, and at 10 ft intervals in-line with the piers upstream and downstream up to 50 ft.

4. Soundings were taken parallel to the bridge at the upstream and downstream fascias, at 10 ft intervals between the



1A CREEK 184 (OTT-2-1312) - UPSTREAM FY, OHIO RUSHA C 6200184 (10NS - UP COUNTY, OVER RE NO. (SECTION

INSPECTED BY: MOR DRAWN BY: BLV CHECKED BY: JMJ DATE: APR 2020

RUSHA CREEK 6200184 (OTT-2-1312) IONS - CENTERLINE SR-2 OVER F STRUCTURE NO. 6 CROSS SECTIO

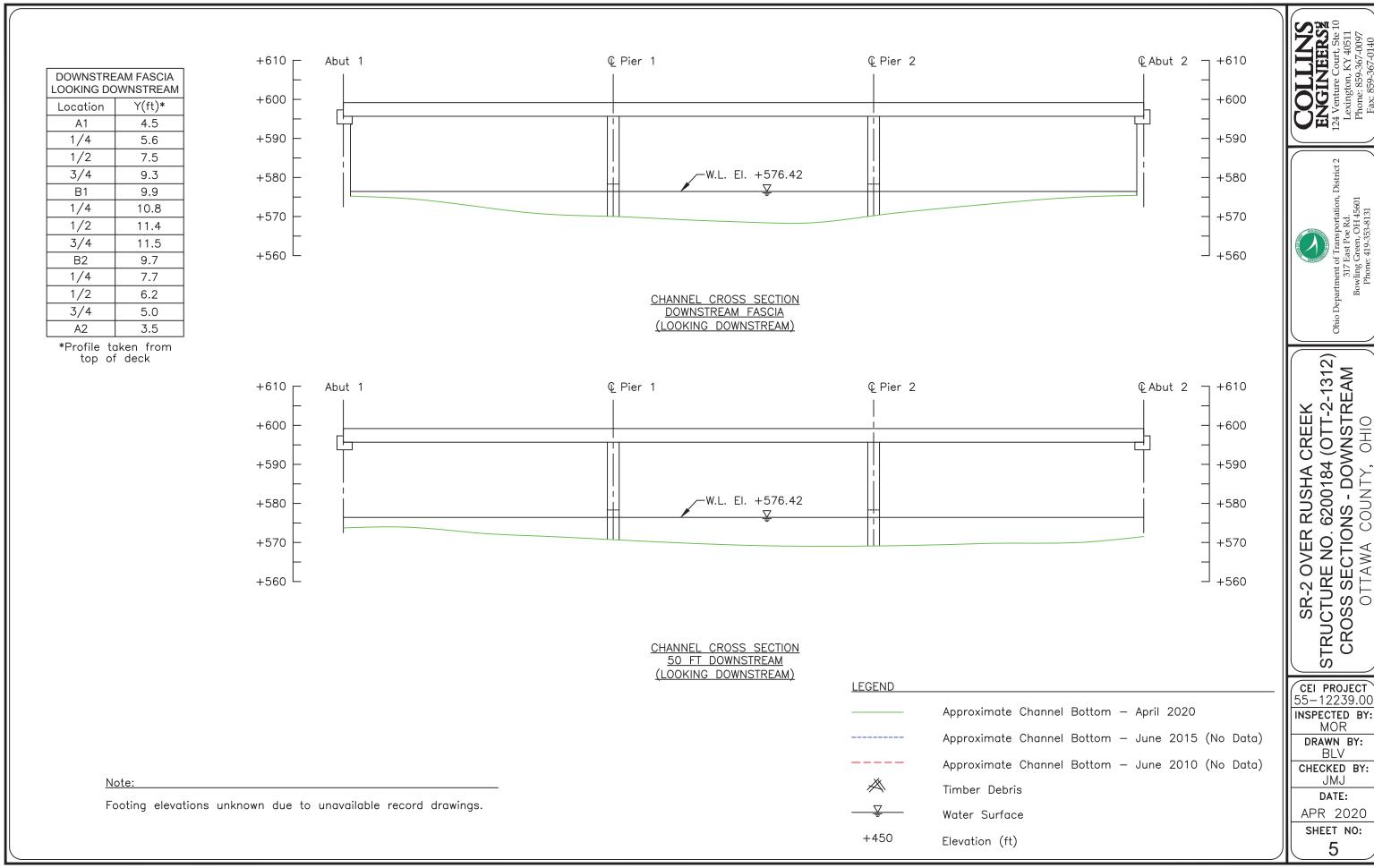
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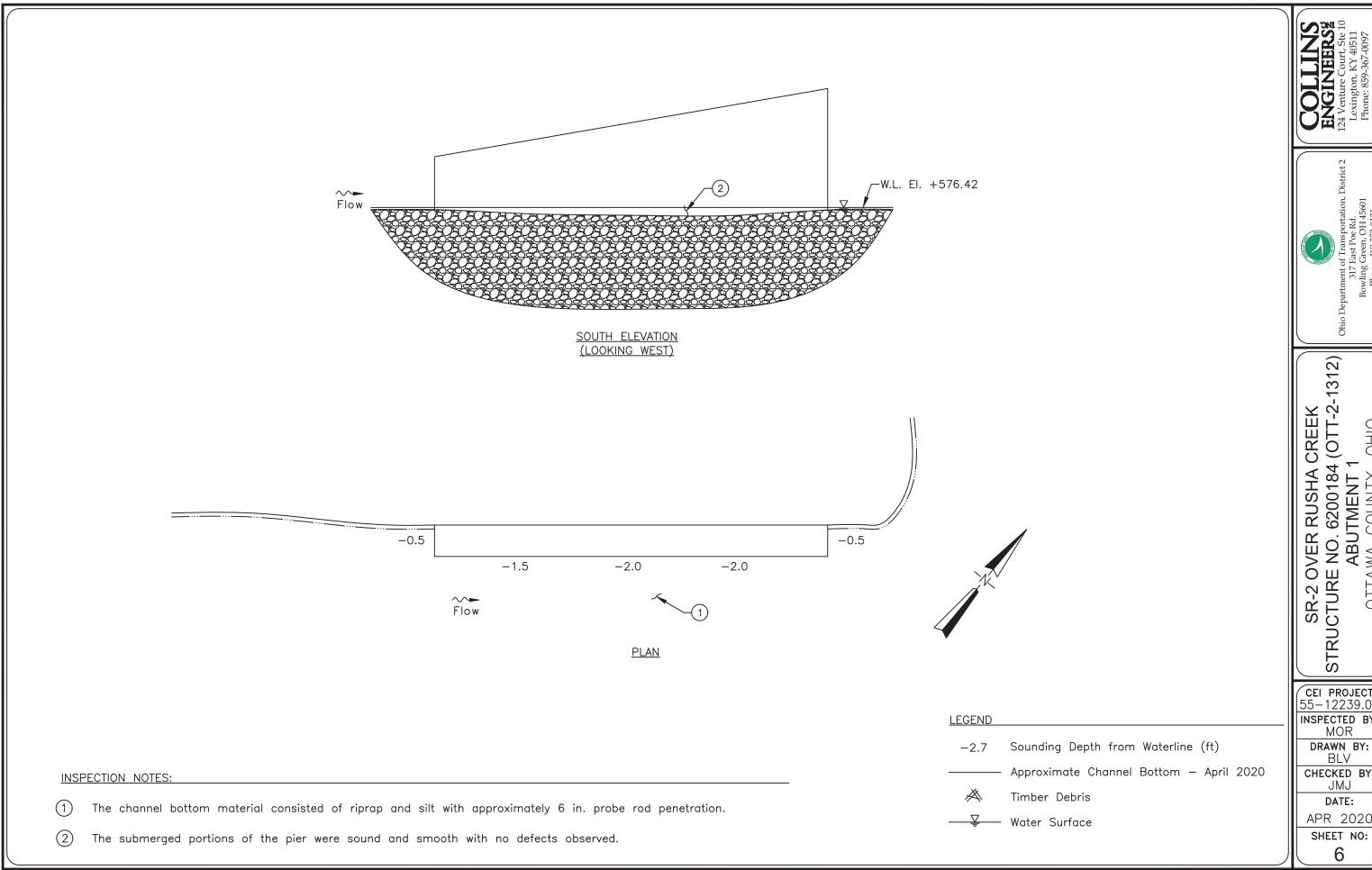
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CHECKED BY: JMJ

DATE:

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CREEK 4 (OTT-2-1312)

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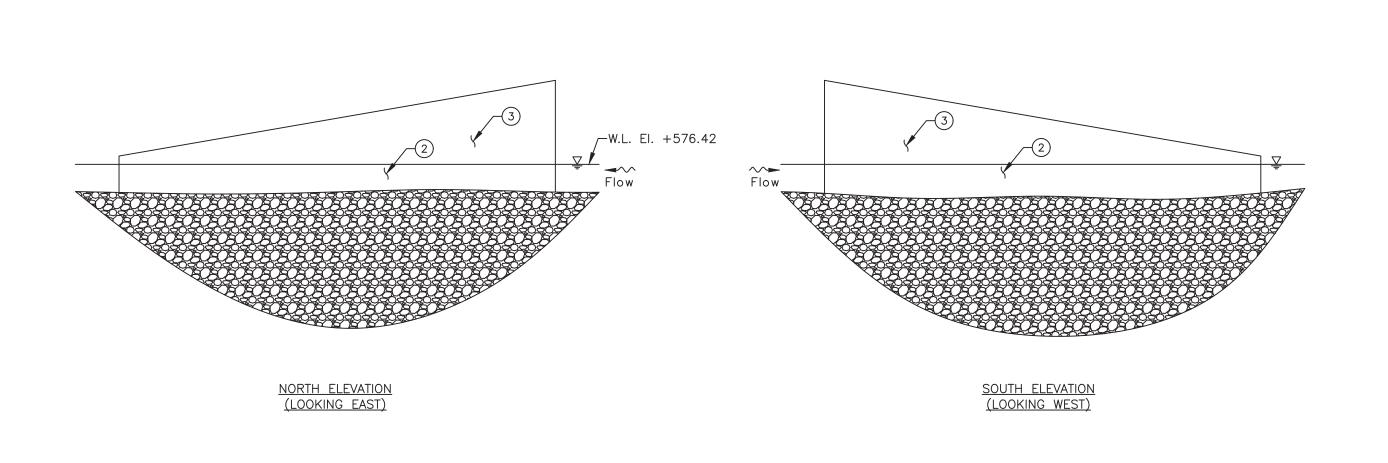
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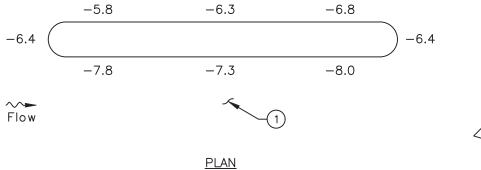
CHECKED BY: JMJ

DATE:

APR 2020

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INSPECTION NOTES:

- 1) The channel bottom material consisted of riprap and silt with approximately 12 in. probe rod penetration.
- 2 The submerged portions of the pier were sound and smooth with no defects observed.
- 3 Vertical cracking measuring 1/16 in. was observed throughout the pier faces spaced 6 ft on center.

LEGEND

-2.7 Sounding Depth from Waterline (ft)

Approximate Channel Bottom — April 2020

Timber Debris

COLLIN ENGINEERS 124 Venture Court, Ste

> nent of Transportation, Distrast Poe Rd. 317 East Poe Rd. Iling Green, OH 45601

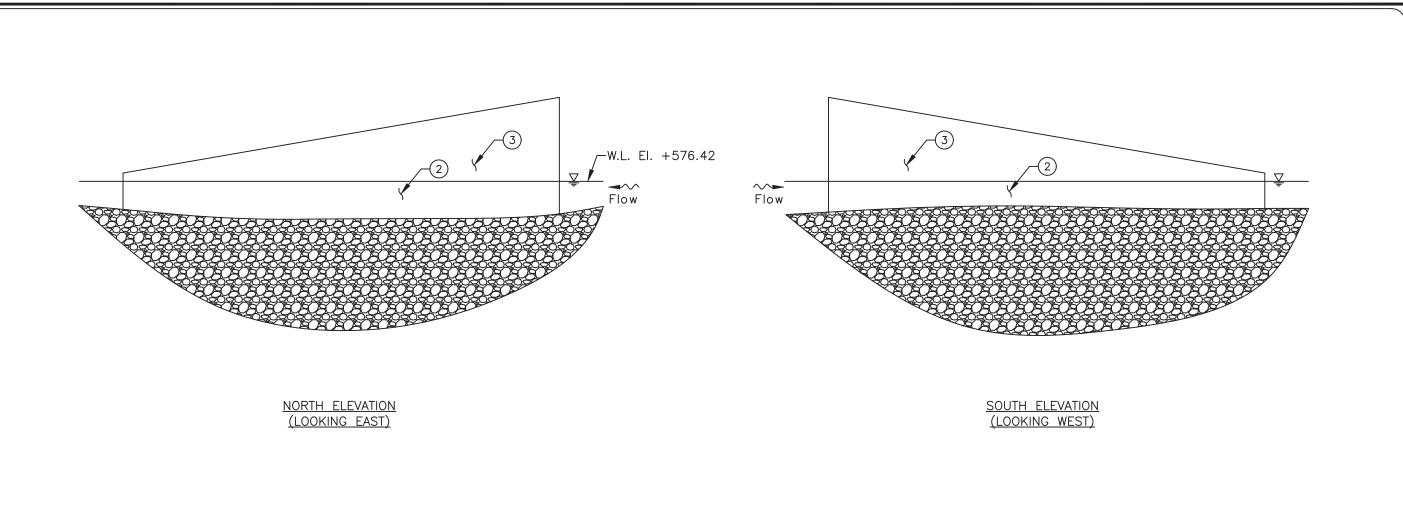
SR-2 OVER RUSHA CREEK STRUCTURE NO. 6200184 (OTT-2-1312) PIER 1

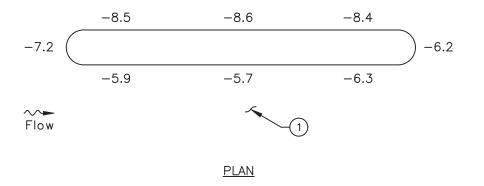
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BLV CHECKED BY: JMJ

DATE:

APR 2020 SHEET NO:







INSPECTION NOTES:

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LEGEND

-2.7 Sounding Depth from Waterline (ft)

Approximate Channel Bottom — April 2020

Timber Debris

COLLIN ENGINEERS 124 Venture Court, Ste.

> urtment of Transportation, Distr 317 East Poe Rd. Sowling Green, OH 45601

SR-2 OVER RUSHA CREEK STRUCTURE NO. 6200184 (OTT-2-1312) PIER 2

CEI PROJECT 55-12239.00 INSPECTED BY: MOR DRAWN BY: BLV

BLV

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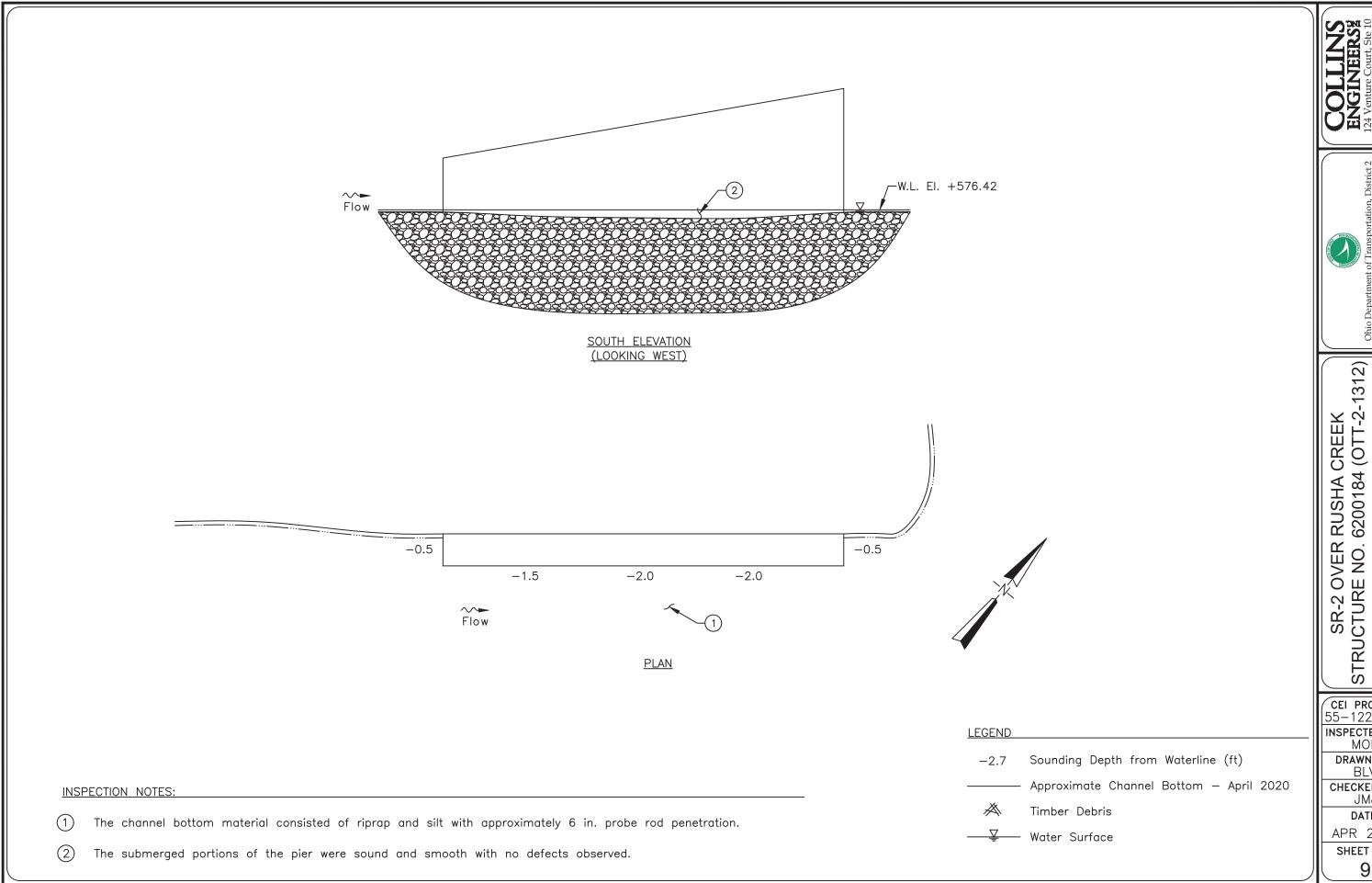
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EXHIBIT 2 – INSPECTION PHOTOGRAPHS







Photograph No. 1: Overall View of Structure No. 6200184 (OTT-2-1312), Looking Southwest.



Photograph No. 2: Overall View of Structure No. 6200184 (OTT-2-1312), Looking Northeast.







Photograph No. 3: View of the West Embankment Upstream of the Structure, Looking West.



Photograph No. 4: View of the West Embankment at the Structure, Looking North.







Photograph No. 5: View of the West Embankment at the Structure, Looking North.



Photograph No. 6: View of the West Embankment Downstream of the Structure, Looking West.







Photograph No. 7: View of the East Embankment Upstream of the Structure, Looking South.



Photograph No. 8: View of the East Embankment at the Structure, Looking East.







Photograph No. 9: View of the East Embankment Downstream of the Structure, Looking South.



Photograph No. 10: View of the North Face of Pier 1, Looking South.







Photograph No. 11: View of the South Face of Pier 1, Looking North.



Photograph No. 12: View of the North Face of Pier 2, Looking South.







Photograph No. 13: View of the South Face of Pier 2, Looking North.



Photograph No. 14: View of the Typical Concrete Condition at the Waterline, Looking North.



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EXHIBIT 3 – UNDERWATER DIVE INSPECTION PROCEDURE CHECKLIST



Underwater Dive Inspection Procedure Checklist

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a standalone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

I. Bridge Identification

a. Agency with Inspection Responsibility: ODOT DISTRICT 2

Dive Frequency: <u>60 months</u>

SFN: <u>6200184</u> Bridge Number (County-Route-SLM-SD): <u>OTT-2-1312</u>

Superstructure Type Main Span Type: <u>CONTINUOUS CONCRETE SLAB</u>

Approach Span: REINFORCED CONCRETE

Substructure Type Abutment Type: REINFORCED CONCRETE

Pier Type: REINFORCED CONCRETE BENTS

Total Pier Count: 2

Total Pier Count in water: 2

Foundations: <u>UNKNOWN</u>

Feature Intersected RUSHA CREEK

b. Photographs

Endview



Elevation



Underside

II. Office and Field Assessment

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

a. Channel Conditions	b. Anticipated Water conditions which
Waterway features	may affect the inspection
Rapid stream flows,	Cold Water (Apprx. Temp)
Significant debris accumulation	Black water
Constricted waterway openings	Rapid stream flows
Soft or unstable streambeds	Near military facility
Meandering channels	Tribal fishing
Other which may promote scour and	Water quality
undermining of substructure elements	History of Log jams
Navigable Waterway	
Flow Controls	c. Identify factors that may accelerate the
	deterioration of the bridge elements:
	Highly corrosive water
	Unprotected steel members
	Other
Risk Factor Narrative:	

III. Contacts Prior to Work

District 2 Bridge Engineer: David Geckle, P.E.				
Email: david.geckle@dot.ohio.gov – Phone: 419-373-4377				
Point of contact for immediate action such as closing the bridge due to findings				
Contact Bridge Owner14 (number) days before the proposed underwater inspection.				
Special contracting and scheduling procedures prior to inspection, include recommended lead time				

Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard			
Property Owner			
Access Equipment			
Lake or River draw- down			
Canal dry time			
Tree removal			
Other:			
Other:			

IV. <u>Dive Team Shall Include the Following:</u>

Dive Team Narrative:					
The dive team consisted of one Team Leader (NBIS, P.E., ADCI) and two Team Members (NBIS, UW, ADCI).					
Example: The Bridge shall be investigated using a three-member dive team: one supervisor to monitor rack box and take notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.					
V. <u>Site Information</u>					
Navigable waterway:	Y / <u>N</u>	Anticipated current <u>0</u> ft			
If Yes, waterway river point		Scour Critical (item 113):5			
Anticipated water visibility deptl	h <u>0</u> ft	POA in place: Y/ <u>N</u>			
Anticipated Dive depth	<u>8.6</u> ft	Scour Monitoring devices present: Y/N			
Verify the Scope of Services when work is contracted for the procedure for underwater elements that					
are not in water during an inspection.					
Site Information Narrative:					

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with
		Commentary
Piers and Number of Columns	2	100% LEVEL II
Abutment	2	100% LEVEL II
Culvert	N/A	
Scour Countermeasures	N/A	
Fenders or Dolphins	N/A	

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

a. The inspection should be conducted The note taker should work alongside the dive using: team. Chest waders ___Hip waders d. Access to the waterway should be X Diving equipment obtained from the shore (north bank, SCUBA (Note that ADCI Consensus southwest quadrant, driveway 30 yards Standards require communication systems be employed for both SCUBA and north etc.) Surface-Supplied (whether air or mixedgas) dive modes) NORTHWEST CORNER OF STRUCTURE SCUBA with communication X Surface Supplied with e. The maximum depth of the channel is communication typically measured_____ feet from EAST CENTERLINE QUARTER POINT BETWEEN b. The channel bottom should be sounded PIERS 1 AND 2 utilizing Reference Datum: 1.9FT. BELOWTHE TOP OF X Digital fathometer THE PIER AT THE DOWNSTREAM NOSE OF PIER _X__Telescoping survey rod 2 ____ acoustic imaging Soundings should be dictated by the scope of work. When not detailed in the scope they c. During the inspection, the divers should should be repeated from the previous work from soundings. If neither exist then they need to be Shore taken in a grid pattern between substructure X Boat units 100' upstream and 100' downstream. Either

VI.

Equipment and Field Logistics

Created: COLLINS ENGINEERS	Date: <u>09/25/2020</u>
Updated By:	Date:
VIII. Other Narrative Not Included In Previo	ous Sections

Inspection Procedure History

VII.