







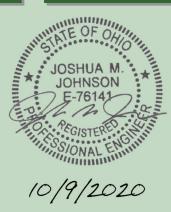
UNDERWATER BRIDGE INSPECTION REPORT

STRUCTURE NO. 6200931 (OTT-19-0811)
SR 19 OVER TOUSSAINT RIVER
OTTAWA COUNTY, OH
DISTRICT 2

May 2020

Prepared for:





Prepared by:

COLLINS ENGINEERS &

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Lexington, Kentucky 40511

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SR 19 over Toussaint River • Structure No. 6200931 (OTT-19-0811) Ottawa County, OH • May 2020



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EXECUTIVE SUMMARY

Project: ODOT District 2 Underwater Bridge Inspections - 2020

Purpose of Project: To perform a detailed visual and tactile underwater investigation of scour critical

bridges for District 2 of the Ohio Department of Transportation.

Inspection Team: Team Leader – Joshua Johnson, P.E. – Collins Engineers, Inc.

Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc.

Team Member – Phillip Osborn – Collins Engineers, Inc.

Inspection Date(s): May 27, 2020

Water Visibility: 1 ft Water Velocity: 0 ft/s

Water Temperature: 65 °F *Weather*: Clear – 70 °F

Waterline Elevation: 775.3 ft *Type of Boat:* N/A

Coordinates: 41.575205°N, -83.14498°W

Access Location: South Embankment at the Structure

Dive Mode: Surface Supplied Air

Waterline Reference: 1.6 ft below the bottom of cap at the upstream nose of Pier 2.

Maximum Depth at SSU: 9.2 ft – Downstream Nose of Pier 1

Shoreline Conditions: The north and south shorelines consisted of sparsely vegetated, well-protected,

gentle slopes with no signs of erosion.

Summary of Findings:

• Abutment 1:

- The channel bottom material consisted of riprap up to 18 in. diameter with no probe rod penetration.
- o The submerged portions of the bent were sound and smooth with no defects observed.

• Bent 1:

- The channel bottom material consisted of sand and cobbles with approximately 2 in. probe rod penetration.
- The submerged portions of the bent were sound and smooth with no defects observed.

• Bent 2:

- The channel bottom material consisted of sand and cobbles with approximately 2 in. probe rod penetration.
- o The submerged portions of the bent were sound and smooth with no defects observed.
- O An impact dent exists on the downstream (east) face of the eastern pile of Bent 2, approximately 2 ft below the waterline measuring 3 in. diameter by 1 in. deep.



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• Abutment 2:

- The channel bottom material consisted of riprap up to 18 in. diameter with no probe rod penetration.
- O The submerged portions of the bent were sound and smooth with no defects observed.

Summary of Recommendations:

- Monitor steel condition of submerged piles.
- Monitor concrete condition at the waterline.



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Underwater Inspection Coding:

NBI Ratings:

Item	Description	Coding	Condition
60	Substructure	7 – Good Condition	Minor Denting of Bent Steel
61	Channel	8 – Very Good Condition	No Defects Observed
62	Culvert	N/A	
92B	UW Insp. Frequency	60 Months	
93B	Insp. Date	05 27 20	
113	Scour Critical Bridges	5 – Within Foundation Limits	Stable (Inspector Recommended)

AASHTO National Bridge Element (NBE) Ratings:

					Conditi	on State	
Element #	Description	Units	Total	1	2	3	4
225	Steel Pile	EA	12	12	0	0	0
215	Reinforced Concrete Abutment	LF	100	100	0	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.



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1.0 INTRODUCTION

1.1 <u>Purpose and Scope</u>

This report consists of the results of a detailed underwater investigation performed at the SR 19 Bridge over Toussaint River in Ottawa County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for District 2 of the Ohio Department of Transportation (ODOT) on May 27, 2020. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Obtain channel profile cross sections at the upstream and downstream fascias.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

1.2 General Description of the Structure

Structure No. 6200931 (OTT-19-0811) spans 69 ft, carrying SR 19 over Toussaint River and is approximately 45 ft wide. The bridge superstructure is constructed of three continuous concrete slab spans. The roadway orientation of the longitudinal axis of the bridge is south to north. The substructure units are labeled as Abutments 1 and 2 and Bents 1 and 2. Existing record drawings were available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.



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A three-person team consisting of a professional engineer-diver and team leader (Joshua Johnson, P.E.) and two engineer divers (Matthew Rogers, E.I.T. and Phillip Osborn) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from the shore and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod and pneumofathometer. Soundings were collected at quarter points along the bridge centerline as well as at quarter points along the upstream and downstream fascias and 50 ft fascias. Additional soundings were collected adjacent to Bents 1 and 2 and at 10 feet intervals in-line with the piers, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

2.0 EXISTING CONDITIONS

2.1 General Conditions

At the time of the inspection, the waterline of 6200931 (OTT-19-0811) was located approximately 1.6 ft below the bottom of cap at the upstream nose of Bent 2, which corresponds to a waterline elevation of 775.3 ft. During the inspection, the waterway was flowing at approximately 0 ft per second. The bridge bent skew was consistent with the channel alignment and does not require attention at this time. The north and south shorelines consisted of sparsely vegetated, well-protected, gentle slopes with no signs of erosion. Refer to Photographs 3 through 8 in Exhibit 2 for views of the shorelines near the structure.



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2.2 Substructure Conditions

2.2.1 Abutment 1

The channel bottom material consisted of riprap up to 18 in. diameter with no probe rod penetration. The submerged portions of the pier were sound and smooth with no defects observed. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Abutment 1.

2.2.2 Bent 1

The channel bottom material consisted of sand and cobbles with approximately 2 in. probe rod penetration. The submerged portions of the bent were sound and smooth with no defects observed. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Bent 1.

2.2.3 Bent 2

The channel bottom material consisted of sand and cobbles with approximately 2 in. probe rod penetration. The submerged portions of the pier were sound and smooth with no defects observed. An impact dent exists on the downstream (east) face of the eastern pile of Bent 2, approximately 2 ft below the waterline measuring 3 in. diameter by 1 in. deep. Refer to Figure 8 in Exhibit 1 for detailed inspection notes of Bent 2.

2.2.4 Abutment 2

The channel bottom material consisted of riprap up to 18 in. diameter with no probe rod penetration. The submerged portions of the bent were sound and smooth with no defects observed. Refer to Figure 9 in Exhibit 1 for detailed inspection notes of Abutment 2. Refer to Photograph 7 in Exhibit 2 for views of the typical concrete condition at the waterline.

3.0 EVALUATION AND RECOMMENDATIONS

Overall, the inspected substructure units of Structure No. 6200931 (OTT-19-0811) were in good condition. A comparison of the soundings recorded during the previous inspection on June 25, 2015 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. Although no channel deficiencies were observed, the channel bottom should continue to be monitored during future underwater inspections to verify that localized scour or overall channel degradation is not occurring and that the pier footings remain adequately embedded in the channel bottom.



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It is recommended that the submerged substructure units of Structure No. 6200931 (OTT-19-0811) be next inspected underwater at an interval not to exceed 60 months, no later than May 27, 2020.

Respectfully Submitted,

COLLINS ENGINEERS, INC.

Joshua Johnson, P.E.

Project Manager

Originated by:

Kevin Mitchell, E.I.T.

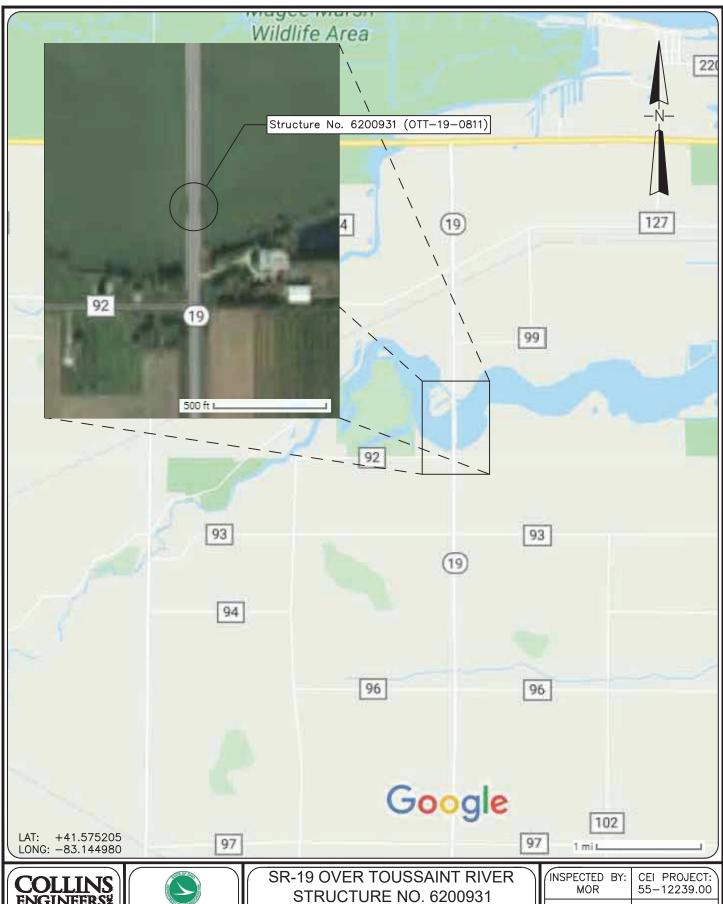


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EXHIBIT 1 – FIGURES





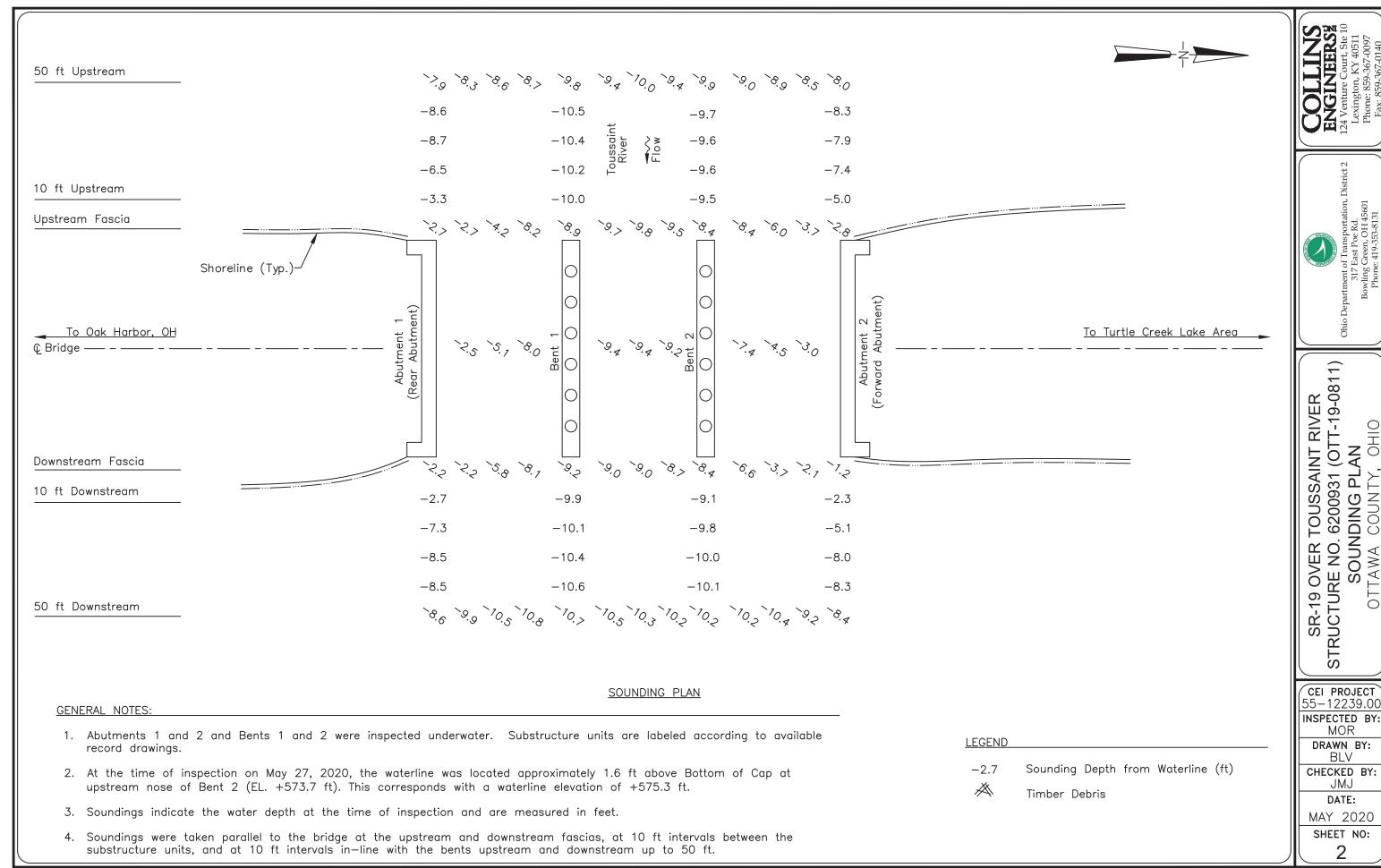
24 Venture Court, Ste 10 Lexington, KY 40511 Phone: 859-367-0097 Fax: 859-367-0140



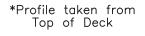
Phone: 419-353-8131

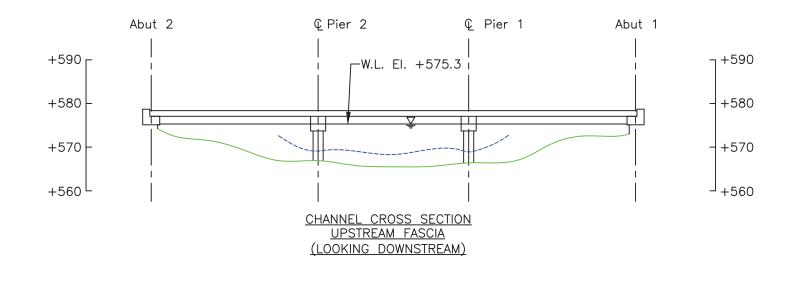
STRUCTURE NO. 6200931 (OTT-19-0811) **LOCATION MAP** OTTAWA COUNTY, OHIO

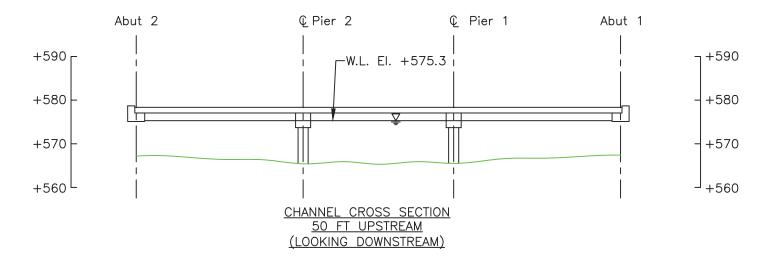
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MOR	55-12239.00
DRAWN BY:	DATE:
BLV	27 MAY 2020
CHECKED BY:	SHEET NO:



UPSTREAM FASCIA LOOKING DOWNSTREAM		
Location	Y(ft)*	
A2	4.1	
1/4	5.0	
1/2	7.3	
3/4	9.7	
P2	9.7	
1/4	10.8	
1/2	11.1	
3/4	11.0	
P1	10.2	
1/4	9.5	
1/2	5.5	
3/4	4.0	
A1	4.0	







LEGEND

+450

Elevation (ft)

Approximate Channel Bottom — May 2020

Approximate Channel Bottom — June 2015

Approximate Channel Bottom — June 2010 (No Data)

Note:

Footing elevations unknown due to unavailable design drawings.

Approximate Channel Bottom — June 2010 (No Data)

Timber Debris

Water Surface

COLLINS

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Lexi
01 Pho

artment of Transporta
317 East Poe Rd.
Bowling Green, OH 4.

SR-19 OVER TOUSSAINT RIVER STRUCTURE NO. 6200931 (OTT-19-0811) CROSS SECTIONS - UPSTREAM

CEI PROJECT 55-12239.00 INSPECTED BY: MOR DRAWN BY:

BLV

CHECKED BY:
JMJ

DATE: MAY 2020

SHEET NO:

R TOUSSAINT RIVER
D. 6200931 (OTT-19-0811)
TIONS - CENTERLINE SR-19 OVER 1 STRUCTURE NO. (CROSS SECTION

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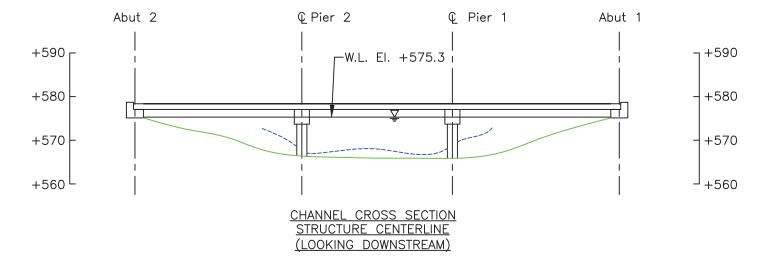
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BLV CHECKED BY: JMJ

DATE:

MAY 2020

SHEET NO: 4



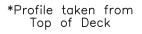
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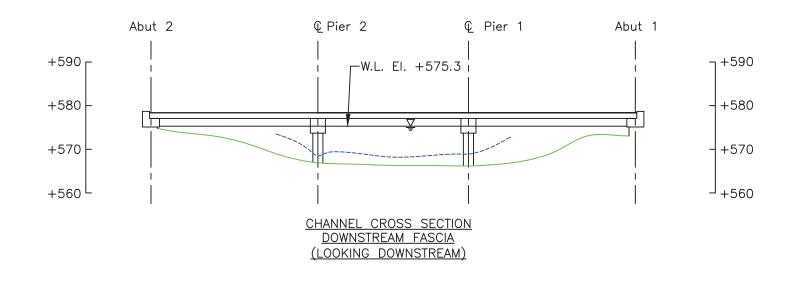
Approximate Channel Bottom — May 2020 Approximate Channel Bottom — June 2015 Approximate Channel Bottom — June 2010 (No Data) Timber Debris

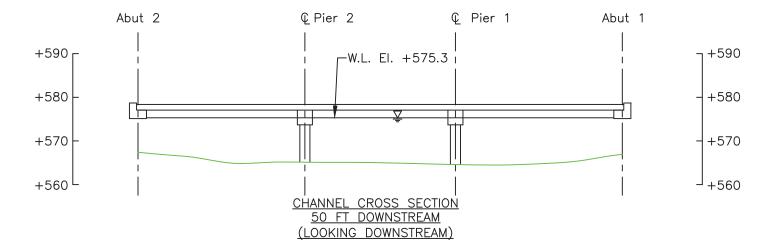
LEGEND

Water Surface

DOWNSTREAM FASCIA LOOKING DOWNSTREAM		
Location	Y(ft)*	
A2	2.5	
1/4	3.4	
1/2	5.0	
3/4	7.9	
P2	9.7	
1/4	10.0	
1/2	10.3	
3/4	10.3	
P1	10.5	
1/4	9.4	
1/2	7.1	
3/4	3.5	
A1	3.5	







LEGEND

Note:
Footing elevations unknown due to unavailable design drawings.

Approximate Channel Bottom — May 2020
Approximate Channel Bottom — June 2015
Approximate Channel Bottom — June 2010 (No Data)

Timber Debris

Water Surface

+450
Elevation (ft)

COLLINS

Transportation, District 2 st Poe Rd. reen, OH 45601

Ohio Department of Transpor 317 East Poe Rc Bowling Green, OH

SR-19 OVER TOUSSAINT RIVER STRUCTURE NO. 6200931 (OTT-19-0811) CROSS SECTIONS - DOWNSTREAM

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55-12239.00
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MOR
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BLV
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JMJ
DATE:
MAY 2020
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R TOUSSAINT RIVER
D. 6200931 (OTT-19-0811)
ABUT 1 SR-19 OVER STRUCTURE NO.

CEI PROJECT 55-12239.00 INSPECTED BY: MOR

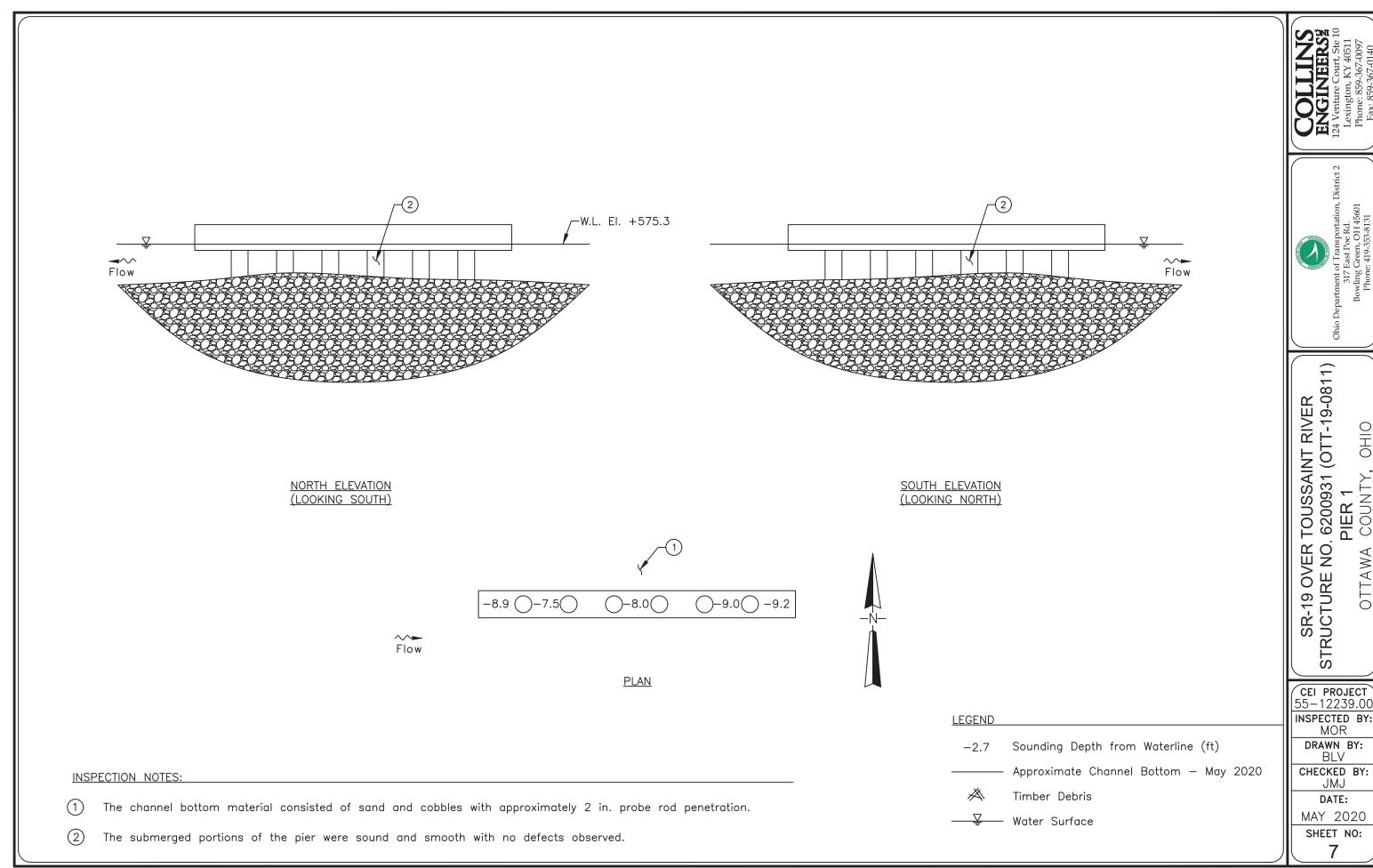
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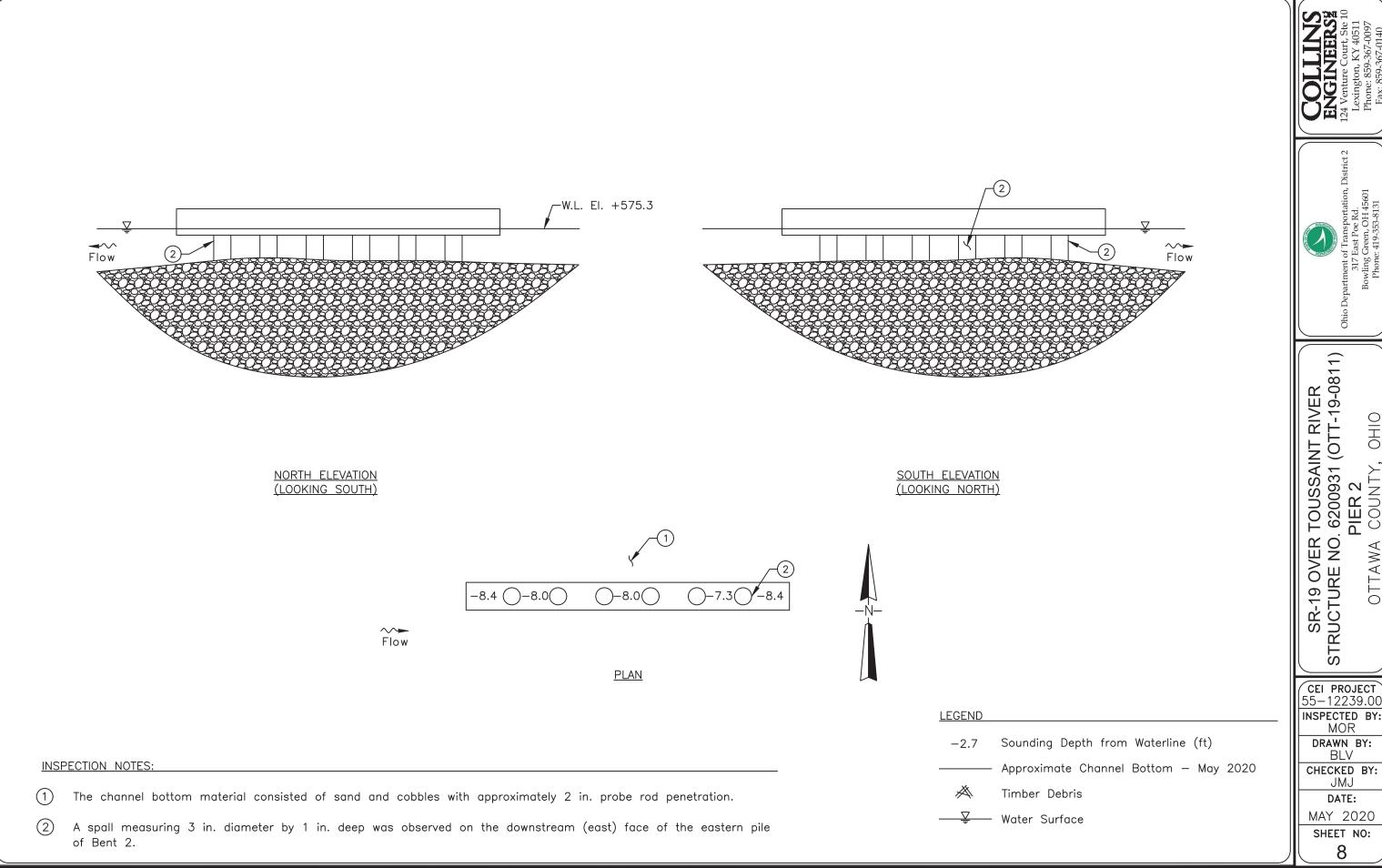
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BLV

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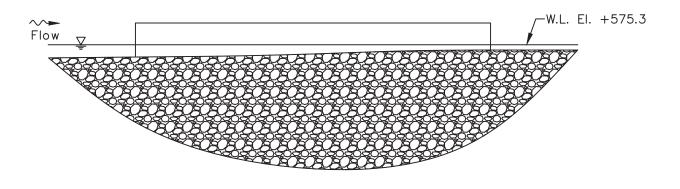
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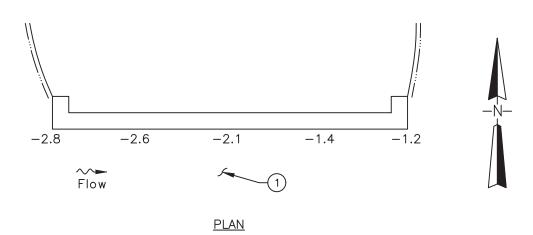
CHECKED BY: JMJ

DATE: MAY 2020

SHEET NO:



SOUTH ELEVATION (LOOKING NORTH)



INSPECTION NOTES:

The channel bottom material consisted of rip—rap up to 18 in. diameter with no probe rod penetration.

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LEGEND

Sounding Depth from Waterline (ft)

Approximate Channel Bottom - May 2020

Timber Debris

— Water Surface

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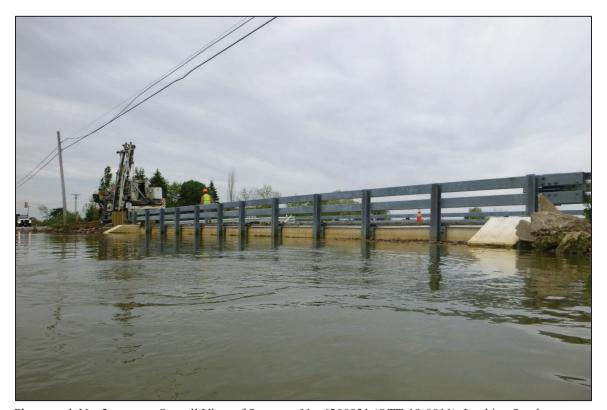
EXHIBIT 2 – INSPECTION PHOTOGRAPHS







Photograph No. 1: Overall View of Structure No. 6200931 (OTT-19-0811), Looking Northeast.



Photograph No. 2: Overall View of Structure No. 6200931 (OTT-19-0811), Looking Southwest.







Photograph No. 3: View of the North Embankment Upstream of the Structure, Looking Northeast.



Photograph No. 4: View of the North Embankment Downstream of the Structure, Looking Northwest.







Photograph No. 5: View of the South Embankment Upstream of the Structure, Looking Southeast.



Photograph No. 6: View of the South Embankment Downstream of the Structure, Looking Southwest.







Photograph No. 7: View of the Typical Concrete Condition at the Waterline, Looking Northwest.

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EXHIBIT 3 – UNDERWATER DIVE INSPECTION PROCEDURE CHECKLIST



Underwater Dive Inspection Procedure Checklist

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a standalone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

I. Bridge Identification

a. Agency with Inspection Responsibility: ODOT DISTRICT 2

Dive Frequency: <u>60 months</u>

SFN: <u>6200931</u> Bridge Number (County-Route-SLM-SD): <u>OTT-19-0811</u>

Superstructure Type Main Span Type: <u>CONTINUOUS CONCRETE SLAB</u>

Approach Span: REINFORCED CONCRETE

Substructure Type Abutment Type: REINFORCED CONCRETE

Pier Type: REINFORCED CONCRETE BENTS

Total Pier Count: 2

Total Pier Count in water: 2

Foundations: <u>UNKNOWN</u>

Feature Intersected <u>TOUSSAINT RIVER</u>

b. Photographs

Endview



Elevation

Underside – N/A

II. Office and Field Assessment

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

a. Channel Conditions	b. Anticipated Water conditions which
Waterway features	may affect the inspection
Rapid stream flows,	Cold Water (Apprx. Temp)
Significant debris accumulation	Black water
Constricted waterway openings	Rapid stream flows
Soft or unstable streambeds	Near military facility
Meandering channels	Tribal fishing
Other which may promote scour and	Water quality
undermining of substructure elements	History of Log jams
Navigable Waterway	
Flow Controls	c. Identify factors that may accelerate the
	deterioration of the bridge elements:
	Highly corrosive water
	Unprotected steel members
	Other
Risk Factor Narrative:	

III. Contacts Prior to Work

District 2 Bridge Engineer: David Geckle, P.E.			
Email: david.geckle@dot.ohio.gov – Phone: 419-373-4377			
Point of contact for immediate action such as closing the bridge due to findings)			
Contact Bridge Owner14 (number) days before the proposed underwater inspection.			
Special contracting and scheduling procedures prior to inspection, include recommended lead time			

			T
Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard			
Property Owner			
Access Equipment			
Lake or River draw-			
down			
Canal dry time			
Tree removal			
Other:			
Other:			

IV. <u>Dive Team Shall Include the Following:</u>

Dive Team Narrative:				
The dive team consisted of one Team Leader (NBIS, P	.E., ADCI) and two Team Members (NBIS, UW, ADCI).			
Example: The Bridge shall be investigated using a three-member dive team: one supervisor to monitor rack box and take notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.				
V. <u>Site Information</u>				
Navigable waterway: Y / N	Anticipated current0_ ft			
If Yes, waterway river point	Scour Critical (item 113):5			
Anticipated water visibility depth <u>1</u> ft	POA in place: Y/ <u>N</u>			
Anticipated Dive depth 9.2 ft	Scour Monitoring devices present: Y/N			
Verify the Scope of Services when work is contracted for the procedure for underwater elements that are not in water during an inspection. Site Information Narrative:				

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with
		Commentary
Piers and Number of	2	100% LEVEL I
Columns		10% LEVEL II
	2	100% LEVEL I
Abutment		10% LEVEL II
Culvert	N/A	
Scour Countermeasures	N/A	
Fenders or Dolphins	N/A	

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

a. The inspection should be conducted	The note taker should work alongside the dive
using:	team.
Chest waders	
Hip waders	d. Access to the waterway should be
X_Diving equipment	obtained from the shore (north bank,
SCUBA (Note that ADCI Consensus	southwest quadrant, driveway 30 yards
Standards require communication systems be employed for both SCUBA and	north etc.)
Surface-Supplied (whether air or mixedgas) dive modes)	SOUTH EMBANKMENT AT THE STRUCTURE
SCUBA with communication	
XSurface Supplied with	e. The maximum depth of the channel is
communication	typically measured feet from
b. The channel bottom should be sounded	50FT. FROM DOWNSTREAM PIER 1
utilizing	Reference Datum: <u>1.6FT. BELOW THE BOTTOM</u>
Digital fathometer	CAP AT THE UPSTREAM NOSE OF PIER 2.
X_Telescoping survey rod	Soundings should be dictated by the scope of
acoustic imaging	work. When not detailed in the scope they
	should be repeated from the previous
c. During the inspection, the divers should	soundings. If neither exist then they need to be
work from	taken in a grid pattern between substructure
_XShore	units 100' upstream and 100' downstream.
Boat	
Either	

Equipment and Field Logistics

VI.

	
Created: COLLINS ENGINEERS	Date: <u>09/25/2020</u>
Updated By:	Date:
VIII. Other Narrative Not Included In Previous Sections	

VII.

Inspection Procedure History