



# **UNDERWATER BRIDGE**

## **INSPECTION REPORT**

STRUCTURE NO. 7202431 (SAN-53-1745) SR 53 OVER MUDDY CREEK SANDUSKY COUNTY, OH DISTRICT 2

May 2020

Prepared for:



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10/9/2020

Prepared by:



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#### UNDERWATER INSPECTION

SR 53 over Muddy Creek • Structure No. 7202431 (SAN-53-1745) Sandusky County, OH • April 2020



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#### EXECUTIVE SUMMARY

Project:	ODOT District 2 Underwater Bridge Inspections - 2020				
Purpose of Project:	To perform a detailed visual and tactile underwater investigation of underwater bridges for District 2 of the Ohio Department of Transportation.				
Inspection Team:	Team Leader – Joshua Johnson, P.E. – Collins Engineers, Inc. Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc. Team Member – Phil Osborn, E.I.T. – Collins Engineers, Inc.				
Inspection Date(s):	May 27, 2020				
Water Visibility:	1 ft	Water Velocity:	<1 ft/s		
Water Temperature:	76 °F	Weather:	Clear – 85 °F		
Waterline Elevation:	572.0 ft	Type of Boat:	23 ft Carolina Skiff		
Coordinates:	41.452057°N, 83.054888°W				
Access Location:	Riverfront Marina & Campgro	unds Public Boat Ramp			
Dive Mode:	Surface Supplied Air				
Waterline Reference:	3.3 ft below the top of	cap at the upstream nose	e of Bent 1.		
Maximum Depth at SS	SU: 14.8 ft – Midpoint of H	Bent 2			
Shoreline Conditions:		The north and south shorelines consisted of moderately vegetated gentle slopes which were well protected with no signs of erosion.			

#### Summary of Findings:

- Bent 1:
  - The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration.
  - The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft.
  - $\circ$  The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap.
- Bent 2:
  - The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration.
  - The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft.
  - $\circ$  The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap.
- Bent 3:
  - The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration.
  - The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft.





• The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap.

#### • Bent 4:

- The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration.
- The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft.
- $\circ$  The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap.

#### Summary of Recommendations:

• Monitor light surface corrosion on steel H-piles.





#### **Underwater Inspection Coding:**

#### **NBI Ratings:**

Item	Description	Coding	Condition
60	Substructure	7 – Good Condition	Light Surface Corrosion
61	Channel	7 – Good Condition	
62	Culvert	N/A	
92B	UW Insp. Frequency	60 Months	
93B	Previous Insp. Date	05/27/20	
113	Scour Critical Bridges	5 – Within Foundation Limits	Stable (Inspector Recommended)

#### AASHTO National Bridge Element (NBE) Ratings:

				Condition State			
Element #	Description	Units	Total	1	2	3	4
225	Steel Pile	EA	36	24	8	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.





#### 1.0 INTRODUCTION

#### 1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the SR 53 Bridge over Sandusky River in Sandusky County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for District 2 of the Ohio Department of Transportation (ODOT) on May 27, 2020. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Obtain channel profile cross sections at the upstream and downstream fascias.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

#### 1.2 <u>General Description of the Structure</u>

Structure No. 7202431 (SAN-53-1745) spans 188 ft, carrying SR 53 over Sandusky River and is approximately 25.0 ft wide. The bridge superstructure is constructed of five continuous reinforced concrete spans. The roadway orientation of the longitudinal axis of the bridge is south to north. The substructure units are labeled as Abutments 1 and 2 and Bents 1 through 4. Existing record drawings were available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

#### 1.3 <u>Method of Investigation</u>

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.





A three-person team consisting of a professional engineer-diver and team leader (Joshua Johnson, P.E.) and two engineer divers (Matthew Rogers, E.I.T. and Phillip Osborn, E.I.T.) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from a boat and a note taker in the boat recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod and digital fathometer. Soundings were collected at quarter points along the bridge centerline as well as at quarter points along the upstream and downstream fascias and 50 ft fascias. Additional soundings were collected adjacent to Bents 1 through 4 and at 10 feet intervals in-line with the bents, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

#### 2.0 EXISTING CONDITIONS

#### 2.1 <u>General Conditions</u>

At the time of the inspection, the waterline of 7202431 (SAN-53-1745) was located approximately 3.3 ft below the top of cap at the upstream nose of Bent 1, which corresponds to a waterline elevation of 572.0 ft. During the inspection, the waterway was flowing at less than 1 ft per second. The bridge bent skew was consistent with the channel alignment and does not require attention at this time. The north and south shorelines consisted of moderately vegetated gentle slopes which were well protected with no signs of erosion. Refer to Photographs 3 through 8 in Exhibit 2 for views of the shorelines near the structure.





#### 2.2 <u>Substructure Conditions</u>

#### 2.2.1 Bent 1

The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration. The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft. The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Bent 1. Refer to Photographs 9 and 10 in Exhibit 2 for views of Bent 1.

#### 2.2.2 Bent 2

The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration. The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft. The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Bent 2. Refer to Photographs 11 and 12 in Exhibit 2 for views of Bent 2.

#### 2.2.3 Bent 3

The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration. The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft. The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap. Refer to Figure 8 in Exhibit 1 for detailed inspection notes of Bent 3. Refer to Photographs 13 and 14 in Exhibit 2 for views of Bent 3.

#### 2.2.4 Bent 4

The channel bottom material consisted of rip-rap up to 24 in. diameter with no probe rod penetration. The submerged portions of the steel H-piles exhibited light surface corrosion with rust nodules up to 2 in. diameter from channel bottom to -2 ft. The upstream and downstream nose pile exhibited loss of coating with typically 1/32 in. pitting with a maximum of 1/8 in. from channel bottom to bottom of cap. Refer to Figure 9 in Exhibit 1 for detailed inspection notes of Bent 4. Refer to Photographs 15 through 18 in Exhibit 2 for views of Bent 4 and typical steel condition at the waterline.





#### 3.0 EVALUATION AND RECOMMENDATIONS

Overall, the inspected substructure units of Structure No. 7202431 (SAN-53-1745) were in good condition. A comparison of the soundings recorded during the previous inspection on June 21, 2015 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. Although no channel deficiencies were observed, the channel bottom should continue to be monitored during future underwater inspections to verify that localized scour or overall channel degradation is not occurring and that the bent footings remain adequately embedded in the channel bottom. The corrosion on the upstream and downstream piles of Bents 1 through 4 is not a structural concern at this time, and as a result, no repairs are recommended.

It is recommended that the submerged substructure units of Structure No. 7202431 (SAN-53-1745) be next inspected underwater at an interval not to exceed 60 months, no later than May 27, 2025.

Respectfully Submitted, COLLINS ENGINEERS, INC.

Joshua Johnson, P.E. Project Manager

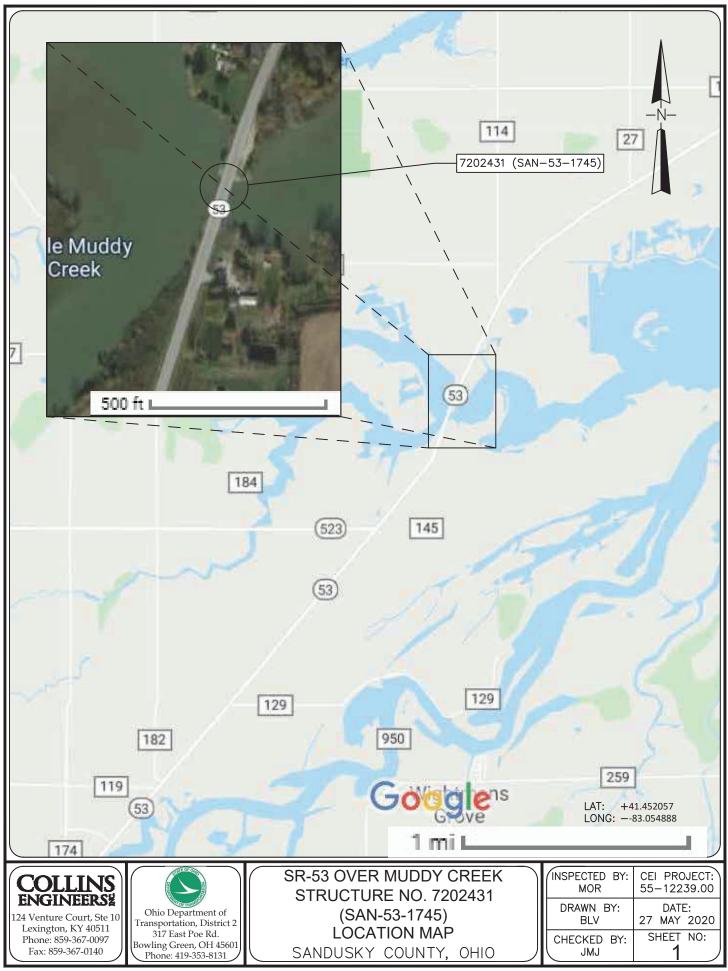
Originated by: Kevin Mitchell, E.I.T.



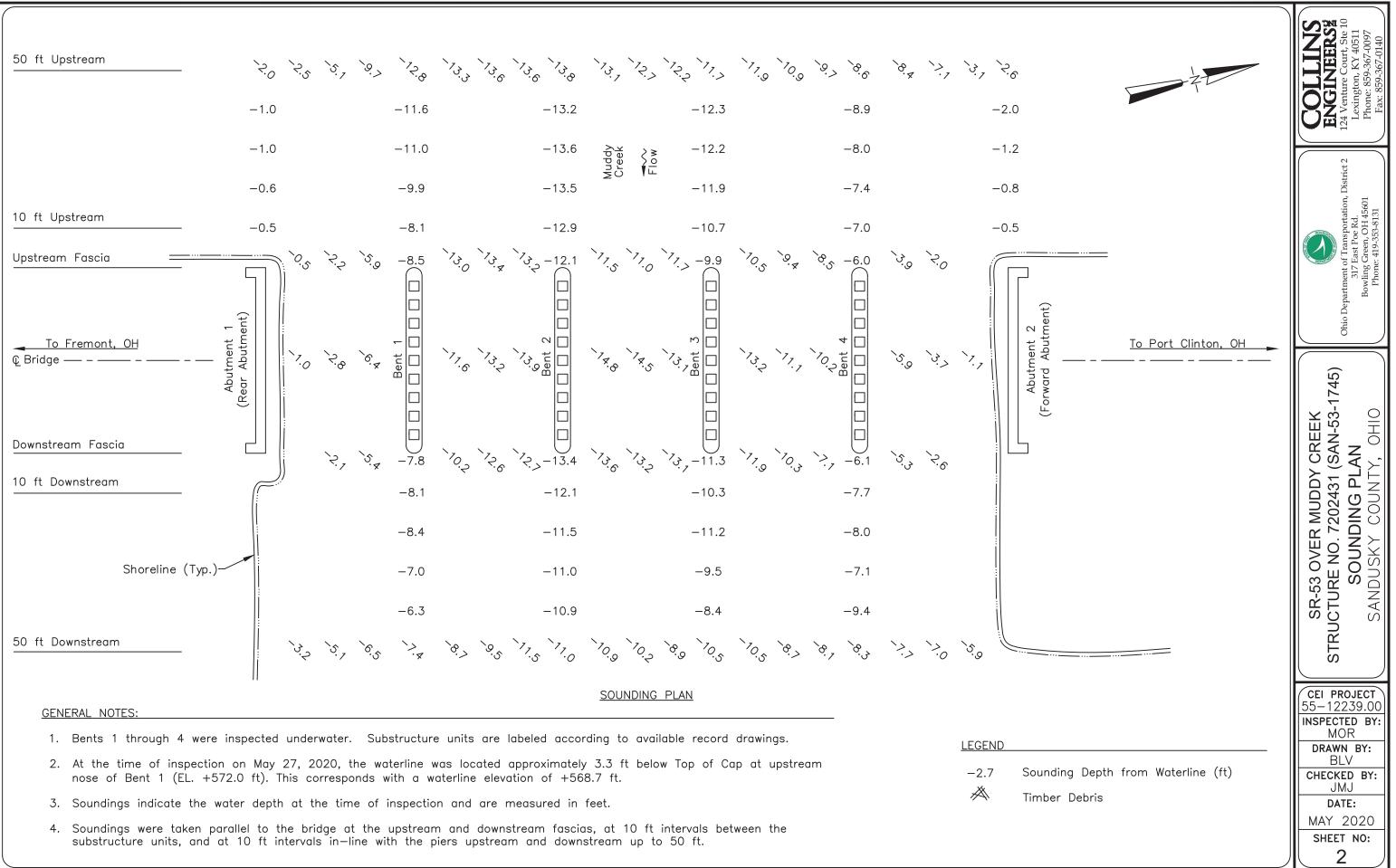


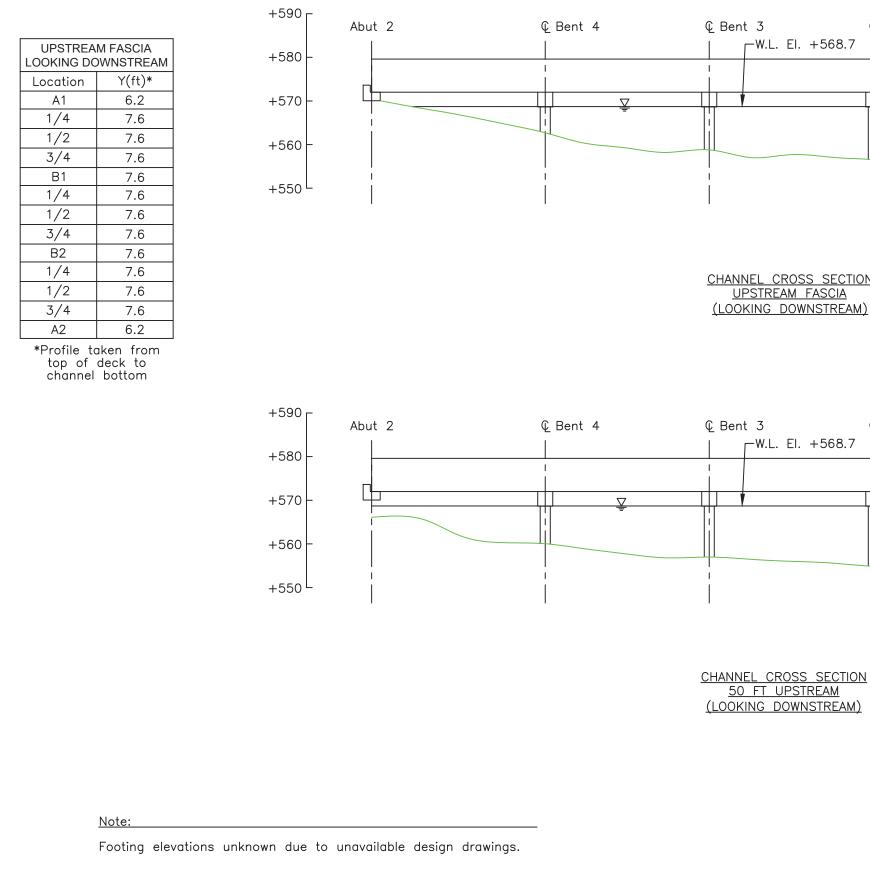
## EXHIBIT 1 – FIGURES





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UPSTREAM FASCIA (LOOKING DOWNSTREAM) ∉ Bent 1

€ Bent 1

€ Bent 2

€ Bent 2

1

LEGEND

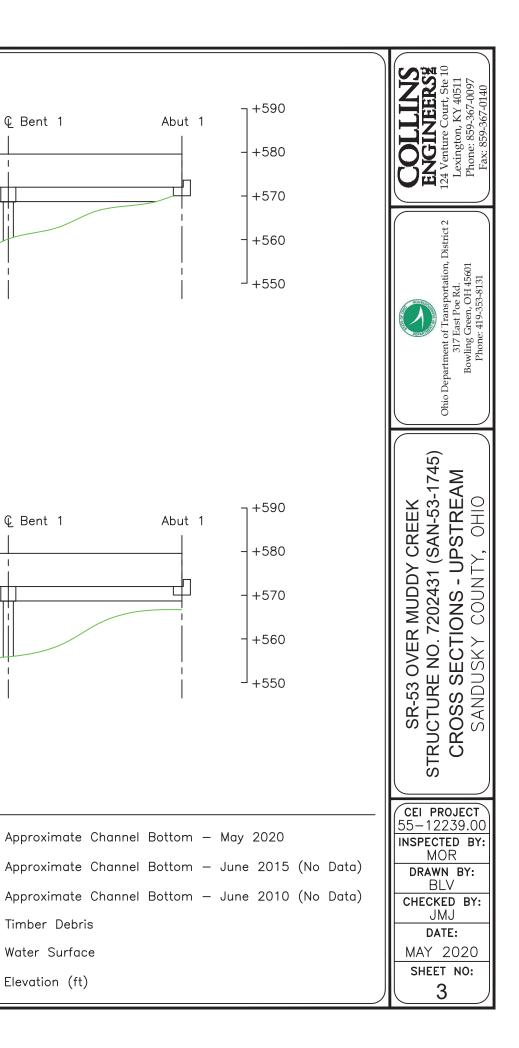
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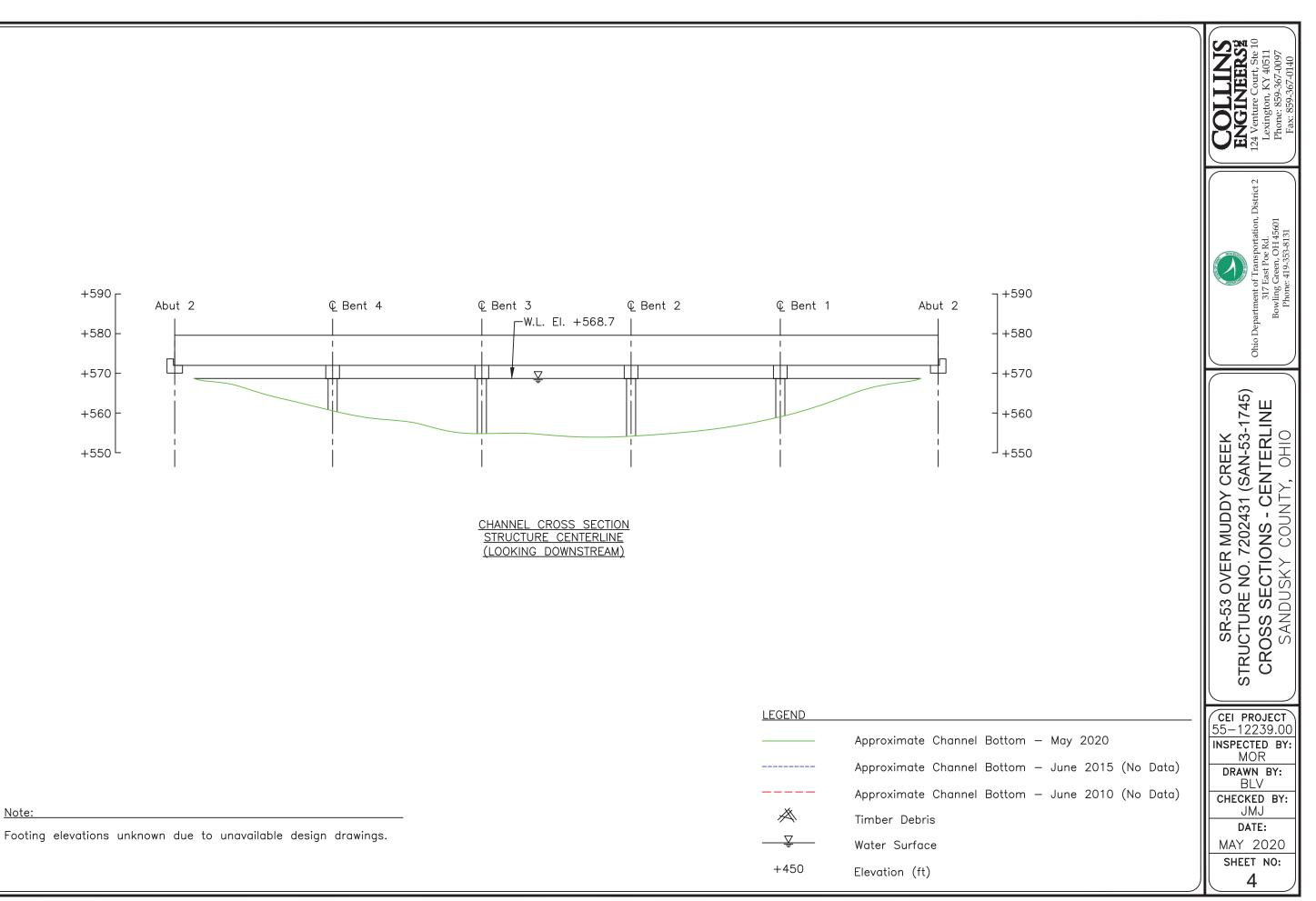
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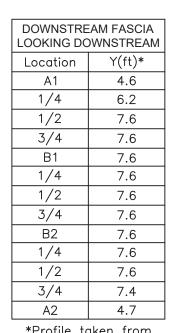
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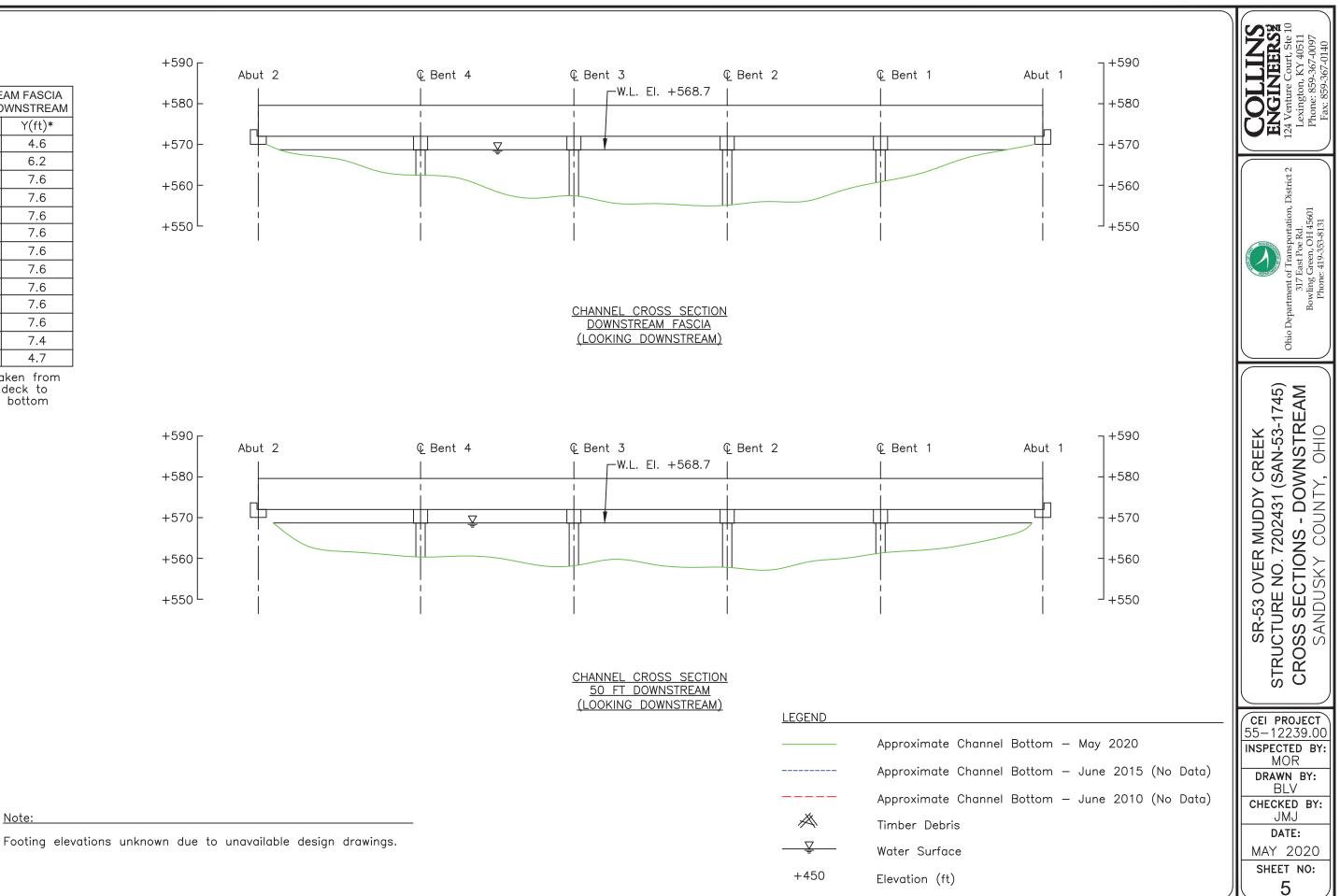


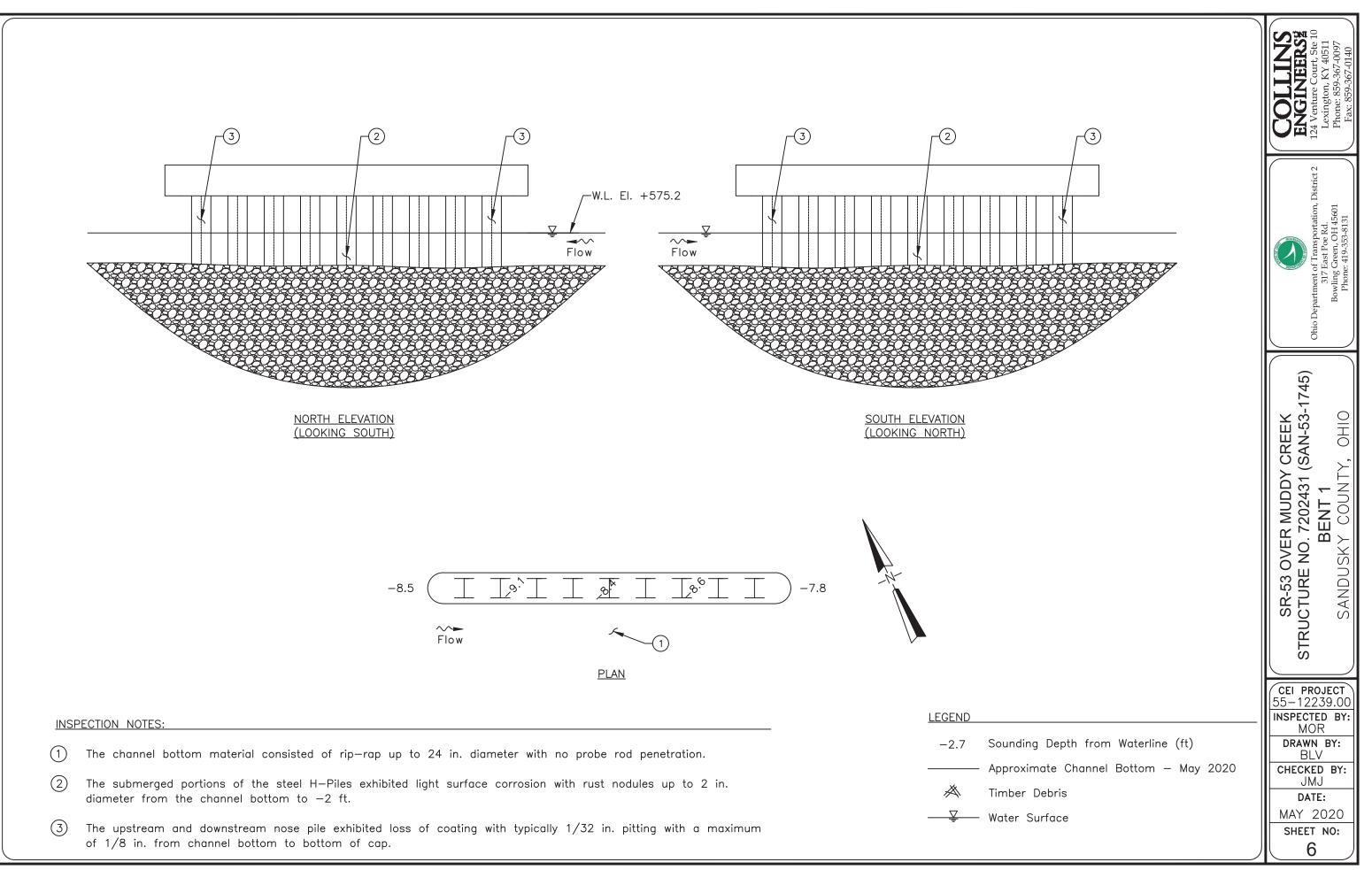


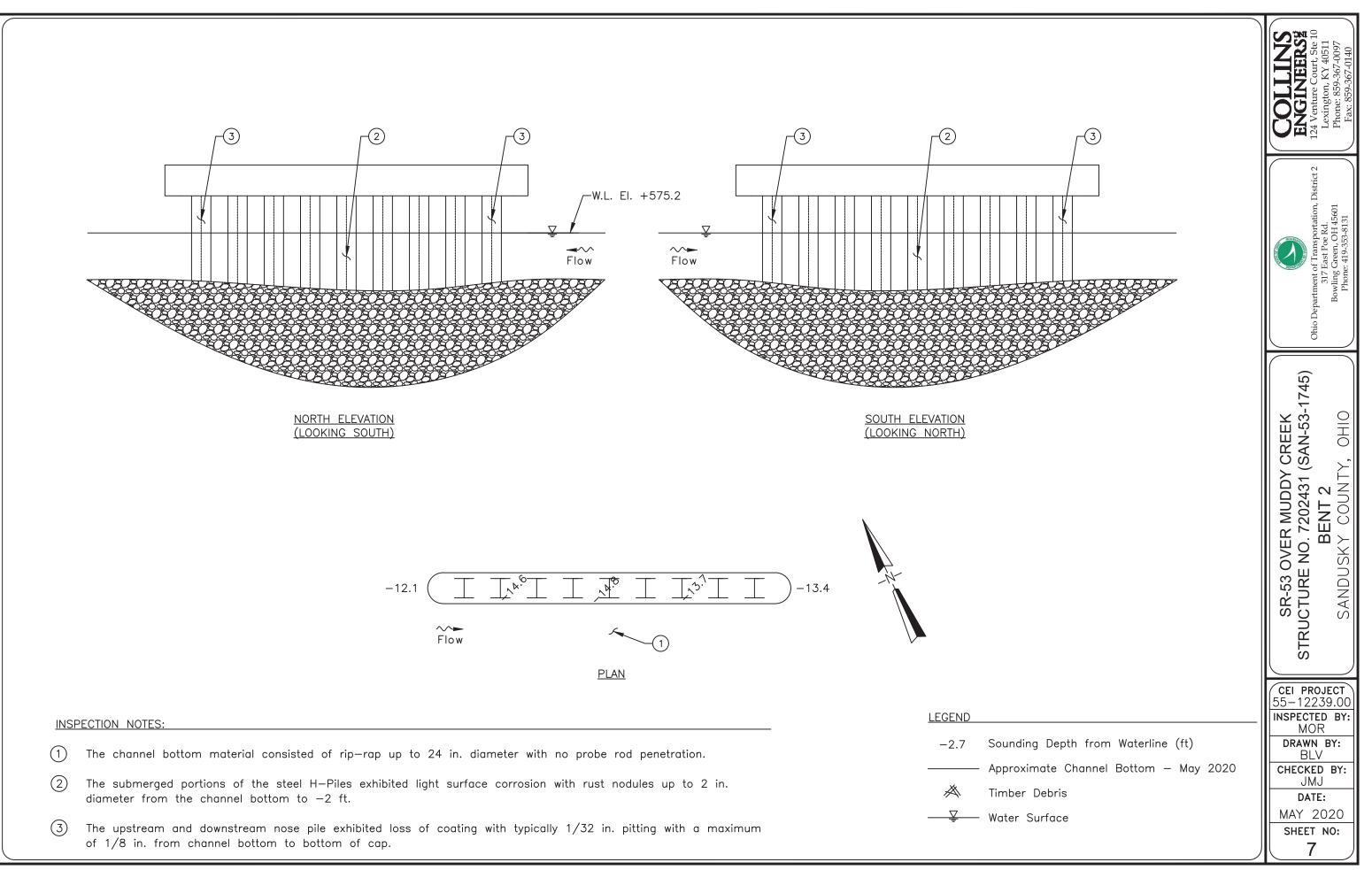


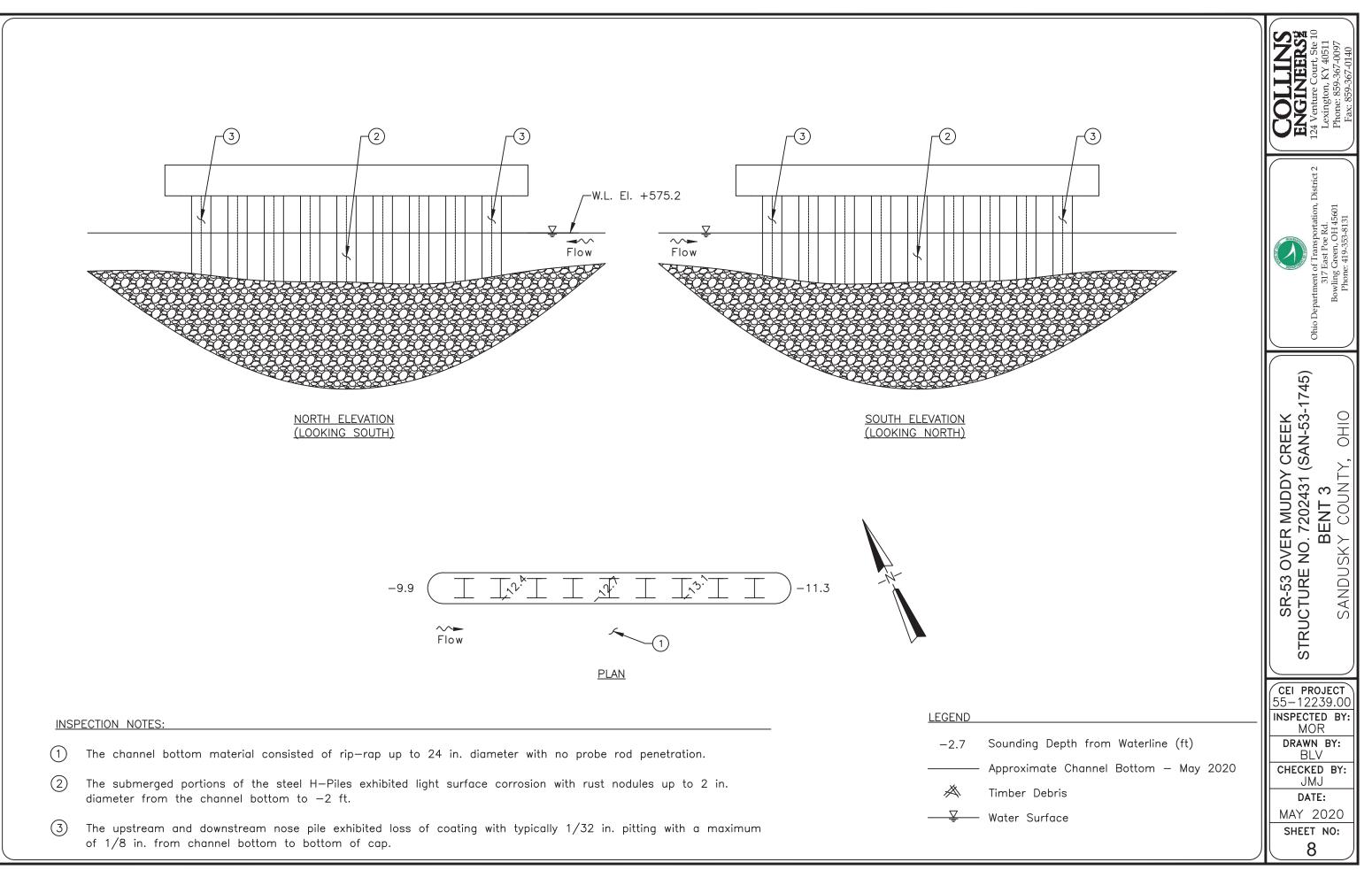
\*Profile taken from top of deck to channel bottom

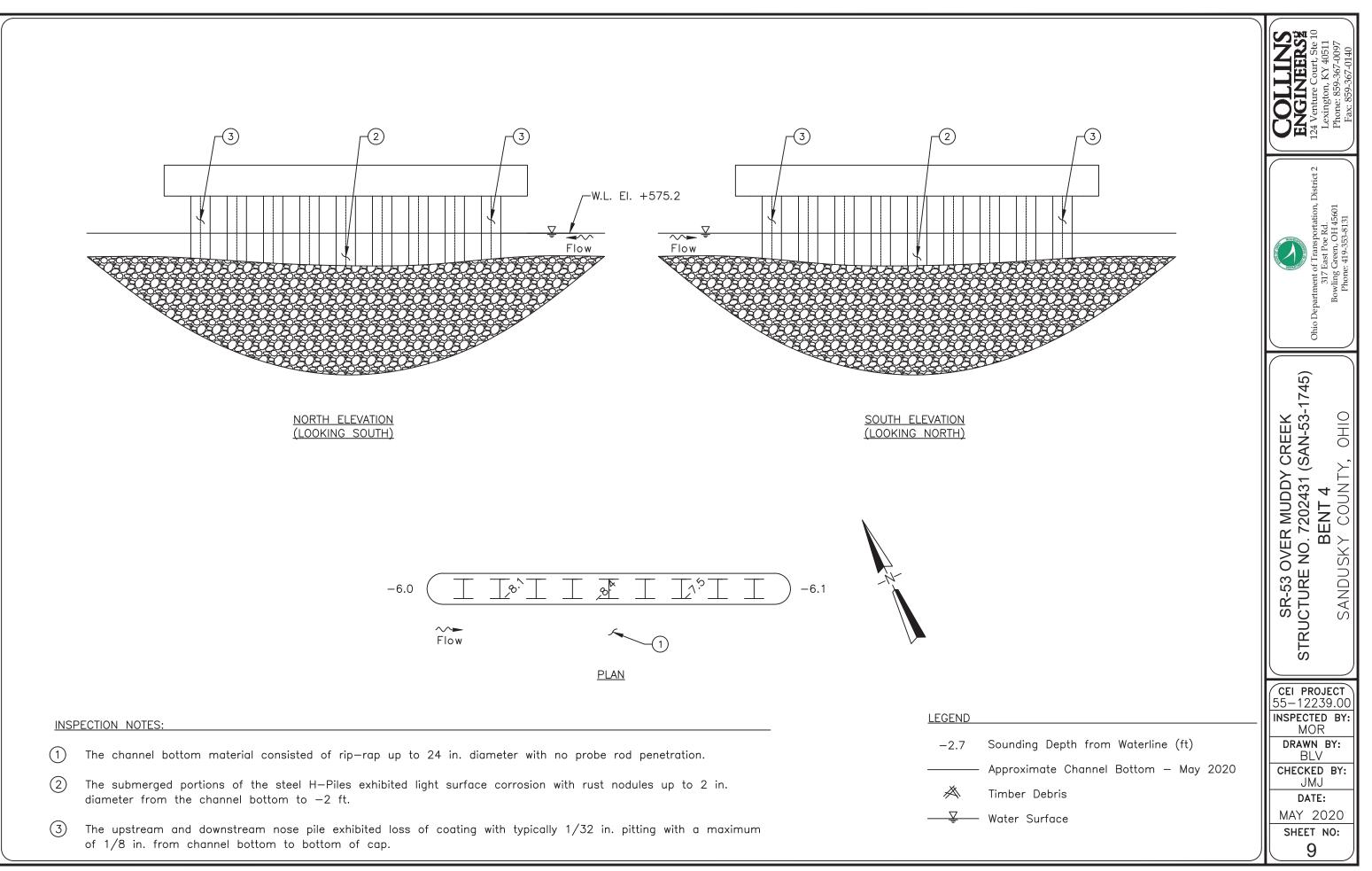
Note:













### **EXHIBIT 2 – INSPECTION PHOTOGRAPHS**







Photograph No. 1: Overall View of Structure No. 7202431 (SAN-5301745), Looking Southeast.



Photograph No. 2: Overall View of Structure No. 7202431 (SAN-53-1745), Looking West.







Photograph No. 3: View of the North Embankment Upstream of the Structure, Looking Northeast.



Photograph No. 4: View of the North Em

View of the North Embankment at the Structure, Looking Northeast.







Photograph No. 5: View of the North Embankment Downstream of the Structure, Looking Northeast.



Photograph No. 6:

View of the South Embankment Upstream of the Structure, Looking Southeast.







Photograph No. 7: View of the South Embankment at the Structure, Looking Southeast.



Photograph No. 8:

View of the South Embankment Downstream of the Structure, Looking Southeast.







Photograph No. 9: View of the North Face of Bent 1, Looking Southeast.



Photograph No. 10:

View of the South Face of Bent 1, Looking Northwest.







Photograph No. 11: View of the North Face of Bent 2, Looking Southeast.



Photograph No. 12: View of the South Face of Bent 2, Looking Northwest.







Photograph No. 13: View of the North Face of Bent 3, Looking Southeast.



Photograph No. 14:

View of the South Face of Bent 3, Looking Northwest.







Photograph No. 15: View of the North Face of Bent 4, Looking Southeast.



Photograph No. 16:

View of the South Face of Bent 4, Looking Northwest.







Photograph No. 17: View of the Typical Steel Condition at the Waterline on the Bent 4, Looking North.



Photograph No. 18:

View of the Typical Steel Condition at the Waterline on the Upstream Nose of Bent 1, Looking South.





### EXHIBIT 3 – UNDERWATER DIVE INSPECTION PROCEDURE

### CHECKLIST



#### **Underwater Dive Inspection Procedure Checklist**

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a standalone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

#### I. Bridge Identification

a.	Agency with Inspection	n Responsibility: ODOT DISTRICT 2	
	Dive Frequency:	60months	
	SFN: <u>7200579</u>	Bridge Number (County-Route-SLM-SD): <u>SAN-53-1745</u>	
	Superstructure Type	Main Span Type:CONTINUOUS REINFORCED CONCRETE	
		Approach Span:REINFORCED CONCRETE	
	Substructure Type	Abutment Type:REINFORCED CONCRETE	
		Pier Type:STEEL H-PILES	
		Total Pier Count:4	
		Total Pier Count in water: <u>4</u>	
		Foundations:UNKNOWN	
	Feature Intersected	MUDDY CREEK	

### b. Photographs





Elevation



Underside

#### II. **Office and Field Assessment**

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

- a. Channel Conditions b. Anticipated Water conditions which \_\_\_\_Waterway features may affect the inspection Rapid stream flows,
  - \_Cold Water (Apprx. Temp\_\_\_\_)

Rapid stream flows

- Significant debris accumulation
- Constricted waterway openings
- Soft or unstable streambeds
- \_\_\_\_Meandering channels
- \_Other which may promote scour and
- undermining of substructure elements

Navigable Waterway

Flow Controls

Near military facility

Black water

- Tribal fishing
- \_Water quality
- \_\_\_\_History of Log jams
- c. Identify factors that may accelerate the

deterioration of the bridge elements:

Highly corrosive water

Unprotected steel members

Other

**Risk Factor Narrative:** 

#### III. Contacts Prior to Work

District 2 Bridge Engineer: David Geckle, P.E.

Email: <u>david.geckle@dot.ohio.gov</u> – Phone: 419-373-4377

Point of contact for immediate action such as closing the bridge due to findings

Contact Bridge Owner \_\_\_\_\_14 (number) days before the proposed underwater inspection.

Special contracting and scheduling procedures prior to inspection, include recommended lead time

Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard			
Property Owner			
Access Equipment			
Lake or River draw- down			
Canal dry time			
Tree removal			
Other:			
Other:			

#### IV. Dive Team Shall Include the Following:

#### Dive Team Narrative:

The dive team consisted of one Team Leader (NBIS, P.E., ADCI) and two Team Members (NBIS, UW, ADCI)

*Example: The Bridge shall be investigated using a three-member dive team: one supervisor to monitor rack box and take notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.* 

#### V. <u>Site Information</u>

Navigable waterway:	Y / <u>N</u>	Anticipated currentft
If Yes, waterway river point	<u>N/A</u>	Scour Critical (item 113): <u>5</u>
Anticipated water visibility depth	n <u>1</u> ft	POA in place: Y/ <u>N</u>
Anticipated Dive depth	<u>15</u> ft	Scour Monitoring devices present: Y/N

Verify the Scope of Services when work is contracted for the procedure for underwater elements that

are not in water during an inspection.

Site Information Narrative:

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with
		Commentary
Piers and Number of	4	100% LEVEL I
Columns		10% LEVEL II
Abutment	N/A	
Culvert	N/A	
Scour Countermeasures	N/A	
Fenders or Dolphins	N/A	

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

#### VI. Equipment and Field Logistics

a. The inspection should be conducted

using:

\_\_\_\_Chest waders

\_\_\_\_Hip waders

<u>X</u> Diving equipment

SCUBA (Note that ADCI Consensus Standards require communication systems be employed for both SCUBA and Surface-Supplied (whether air or mixedgas) dive modes)

\_\_\_\_SCUBA with communication

<u>X</u>Surface Supplied with

- communication
- b. The channel bottom should be sounded

utilizing

- <u>X</u>\_\_\_Digital fathometer
- <u>X</u> Telescoping survey rod
- \_\_\_\_\_ acoustic imaging
- c. During the inspection, the divers should

work from

\_\_\_\_Shore

<u>X</u>Boat

\_\_\_\_Either

The note taker should work alongside the dive team.

d. Access to the waterway should be obtained from the shore (north bank, southwest quadrant, driveway 30 yards north etc.) Riverfront Marina & Campgrounds Public Boat Ramp e. The maximum depth of the channel is typically measured\_\_\_\_\_ feet from The South Quarter Point at the Structure Centerline Between Bents 2 and 3 Reference Datum: Top of Cap at the Upstream Nose of Bent 1 Soundings should be dictated by the scope of work. When not detailed in the scope they should be repeated from the previous soundings. If neither exist then they need to be

taken in a grid pattern between substructure

units 100' upstream and 100' downstream.

### VII. Inspection Procedure History

Created:	COLLINS ENGINEERS, INC	Date:	9/25/2020
Updated By:		Date:	

#### VIII. Other Narrative Not Included In Previous Sections