

# UNDERWATER BRIDGE INSPECTION REPORT

STRUCTURE NO. 7402910 (SEN-224-1207)
US-224 OVER SANDUSKY RIVER
SENECA COUNTY, OH
DISTRICT 2

May 2020

Prepared for:





Prepared by:



**124** Venture Court, Suite 10

Lexington, Kentucky 40511

859.367.0097 • www.collinsengr.com

US-224 over Sandusky River  $\bullet$  Structure No. 7402910 (SEN-224-1207) Seneca County, OH  $\bullet$  May 2020



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#### EXECUTIVE SUMMARY

**Project:** ODOT District 2 Underwater Bridge Inspections - 2020

**Purpose of Project:** To perform a detailed visual and tactile underwater investigation of underwater bridges

for District 2 of the Ohio Department of Transportation.

*Inspection Team:* Team Leader – Joshua Johnson, P.E. – Collins Engineers, Inc.

Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc. Team Member – Phillip Osborn, E.I.T. – Collins Engineers, Inc.

*Inspection Date(s):* May 28, 2020

Water Visibility: <1 ft Water Velocity: 1 ft/s

*Water Temperature:* 78 °F *Weather:* Partly Cloudy – 87 °F

Waterline Elevation: 734.8 ft Type of Boat: Kayak

**Coordinates:** 41.095769°N, -83.199088°W

Access Location: West Shoreline (Days Inn Parking Lot)

**Dive Mode:** Surface Supplied Air

Waterline Reference: 7.5 ft below the top of cap at the downstream nose of Pier 2.
Maximum Depth at SSU: 7.6 ft – Downstream Quarter Point on the North Face of Pier 3

Shoreline Conditions: The east and west shorelines consisted of well protected, well-vegetated, mild

slopes with no signs of erosion and a steel sheet pile wall on the southwest

embankment.

#### Summary of Findings:

#### • Pier 1:

- The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration.
- o The submerged portions of the pier were smooth and south with no defects noted.
- A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the southwest corner of the pier just downstream of the timber debris.
- O Severe timber debris accumulation consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to a 15 ft radius around the upstream nose from channel bottom to 4 ft about the waterline.

#### • Pier 2:

- The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration.
- o The submerged portions of the pier were smooth and south with no defects noted.
- A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the northeast corner of the pier just downstream of the timber debris.
- Heavy timber debris accumulation consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to a 10 ft radius around the upstream nose from channel bottom to 2 ft about the waterline.



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#### • Pier 3:

- The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration.
- o The submerged portions of the pier were smooth and south with no defects noted.
- Moderate timber debris accumulation consisting of logs up to 1 ft diameter were observed on the upstream nose extending along the north face from the channel bottom to 2 ft about the waterline.

#### • Pier 4:

- The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration.
- o The submerged portions of the pier were smooth and south with no defects noted.
- Light timber debris accumulation consisting of branches up to 6 in. diameter were observed on the upstream nose extending up to a 5 ft radius around the upstream nose from the channel bottom to 2 ft about the waterline.

#### Summary of Recommendations:

- Remove timber debris at all Piers.
- Monitor scour during routine underwater inspections.



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#### **Underwater Inspection Coding:**

#### **NBI Ratings:**

<u>Item</u>	Description	Coding	Condition
60	Substructure	7 – Good Condition	No Defects Observed
61	Channel	5 – Fair Condition	Minor Scour, Timber Debris Accumulation
62	Culvert	N/A	
92B	UW Insp. Frequency	60 Months	
93B	Insp. Date	05 28 20	
113	Scour Critical Bridges	8 – Above Foundation Limits	Stable (Inspector Recommended)

#### **AASHTO National Bridge Element (NBE) Ratings:**

				Condition State			
Element #	Description	Units	Total	1	2	3	4
210	Reinforced Concrete Pier Wall	LF	120	120	0	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.



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#### 1.0 INTRODUCTION

#### 1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the US-224 Bridge over Sandusky River in Seneca County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for District 2 of the Ohio Department of Transportation (ODOT) on May 28, 2020. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Obtain channel profile cross sections at the upstream and downstream fascias.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

#### 1.2 General Description of the Structure

Structure No. 7402910 (SEN-224-1207) spans 806.85 ft, carrying US-224 over Sandusky River and is approximately 95 ft wide. The bridge superstructure is constructed of five steel beam spans. The roadway orientation of the longitudinal axis of the bridge is west to east. The substructure units are labeled as Abutments 1 and 2 and Piers 1 through 4. Existing record drawings were available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

#### 1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.



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A three-person team consisting of a professional engineer-diver and team leader (Joshua Johnson, P.E.) and two engineer divers (Matthew Rogers, E.I.T. and Phillip Osborn, E.I.T.) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from a kayak and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod, digital fathometer, and pneumofathometer. Soundings were collected at quarter points along the bridge centerline as well as at quarter points along the upstream and downstream fascias and 50 ft fascias. Additional soundings were collected adjacent to Piers 1 through 4 and at 10 feet intervals in-line with the piers, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

#### 2.0 EXISTING CONDITIONS

#### 2.1 General Conditions

At the time of the inspection, the waterline of 7402910 (SEN-224-1207) was located approximately 7.5 ft below the top of cap at the downstream nose of Pier 2, which corresponds to a waterline elevation of 734.8 ft. During the inspection, the waterway was flowing at approximately 1 ft per second. The bridge pier skew was consistent with the channel alignment and does not require attention at this time. The east and west shorelines consisted of well protected, well-vegetated, mild slopes with no signs erosion and a steel sheet pile wall on the southwest embankment. Refer to Photographs 3 through 8 in Exhibit 2 for views of the shorelines near the structure.



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#### 2.2 <u>Substructure Conditions</u>

#### 2.2.1 Pier 1

The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration. The submerged portions of the pier were smooth and south with no defects noted. A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the southwest corner of the pier just downstream of the timber debris. Severe timber debris accumulation consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to a 15 ft radius around the upstream nose from channel bottom to 4 ft about the waterline. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Pier 1. Refer to Photographs 9 and 10 in Exhibit 2 for views of Pier 1.

#### 2.2.2 Pier 2

The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration. The submerged portions of the pier were smooth and south with no defects noted. A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the northeast corner of the pier just downstream of the timber debris. Heavy timber debris accumulation consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to a 10 ft radius around the upstream nose from channel bottom to 2 ft about the waterline. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Pier 2. Refer to Photographs 11 and 12 in Exhibit 2 for views of Pier 2.

#### 2.2.3 Pier 3

The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration. The submerged portions of the pier were smooth and south with no defects noted. Moderate timber debris accumulation consisting of logs up to 1 ft diameter were observed on the upstream nose extending along the north face from the channel bottom to 2 ft about the waterline. Refer to Figure 8 in Exhibit 1 for detailed inspection notes of Pier 3. Refer to Photographs 13 and 14 in Exhibit 2 for views of Pier 3.

#### 2.2.4 Pier 4

The channel bottom material around the pier consisted of timber debris with twigs and silt overly with up to 6 in. probe rod penetration. The submerged portions of the pier were smooth and south with no defects noted. Light timber debris accumulation consisting of branches up to 6 in. diameter were observed on the upstream nose extending up to a 5 ft radius around the upstream nose from the channel bottom to 2 ft about the waterline.



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Refer to Figure 9 in Exhibit 1 for detailed inspection notes of Pier 4. Refer to Photographs 15 through 17 in Exhibit 2 for views of Pier 4 and typical concrete condition at the waterline.

#### 3.0 EVALUATION AND RECOMMENDATIONS

Overall, the submerged substructure units of Structure No. 7402910 (SEN-224-1207) were in good condition below water. A comparison of the soundings recorded during the previous inspection on June 24, 2015 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. The minor scour depressions observed at Piers 1 through 4 are not a concern at this time given that the bottom of the footing is embedded in the channel bottom. The channel bottom configuration should continue to be closely monitored during future underwater inspections to verify that the scour depressions at Piers 1 through 4 are not increasing and that all footings remain adequately embedded in the channel bottom.

The timber debris accumulations at Piers 1 through 4 is obstructing channel flow, is the cause of scour, and should be removed at this time. Removal of the timber debris will reduce excessive lateral loads on the pier, limit further debris accumulation, and reduce the likelihood of channel bottom degradation resulting from obstructed flow.

It is recommended that the submerged substructure units of Structure No. 7402910 (SEN-224-1207) be next inspected underwater at an interval not to exceed 60 months, no later than May 28, 2025.

Respectfully Submitted,

COLLINS ENGINEERS, INC.

Joshua Johnson, P.E.

Project Manager

Originated by:

Kevin Mitchell, E.I.T.

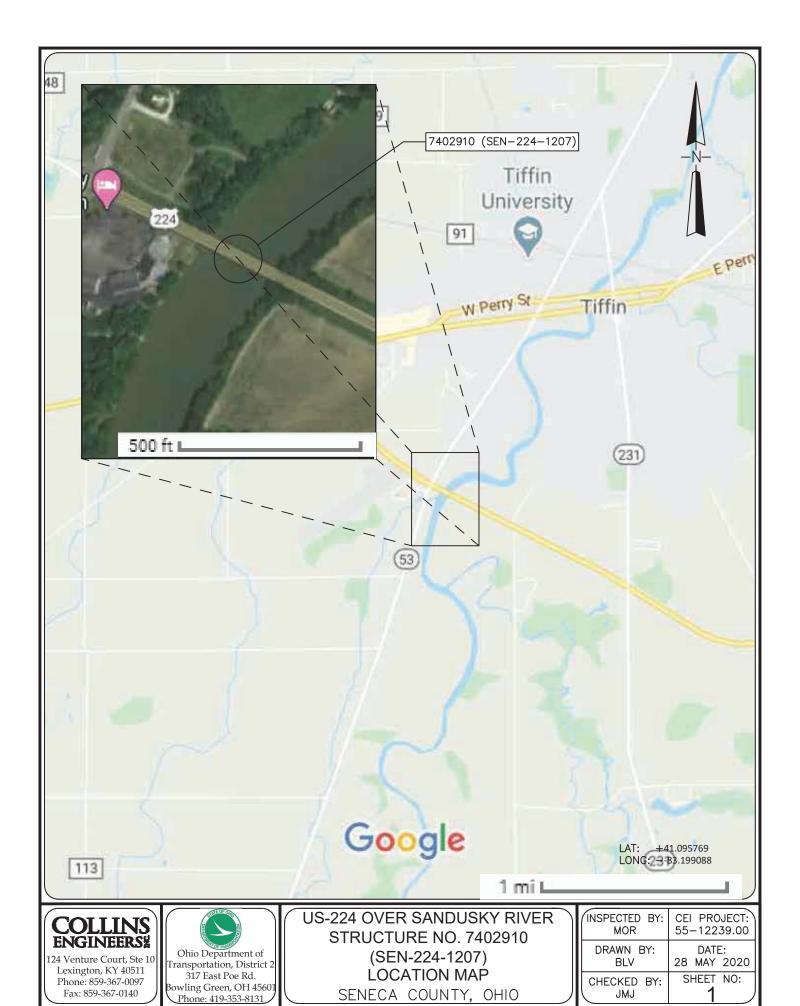


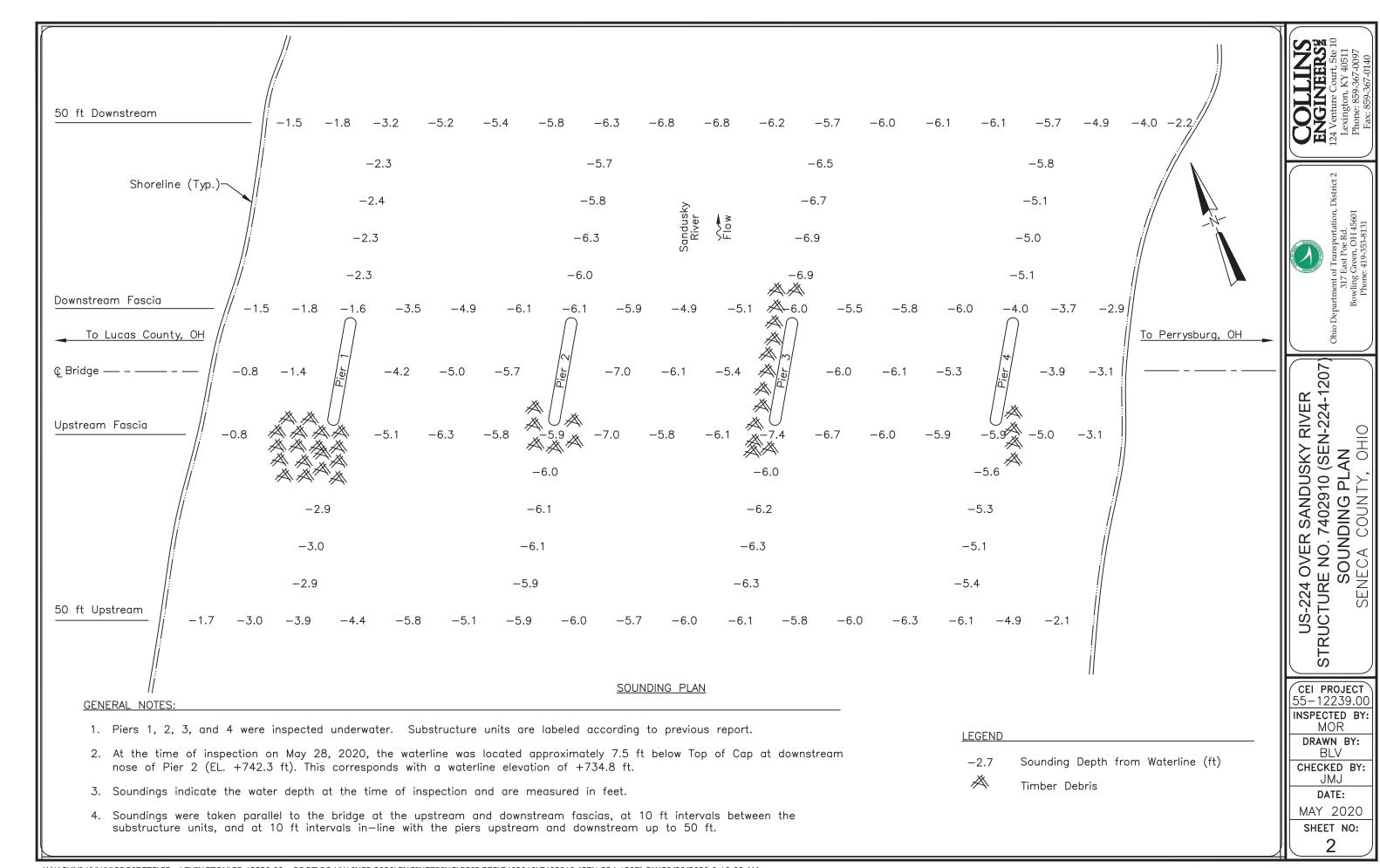
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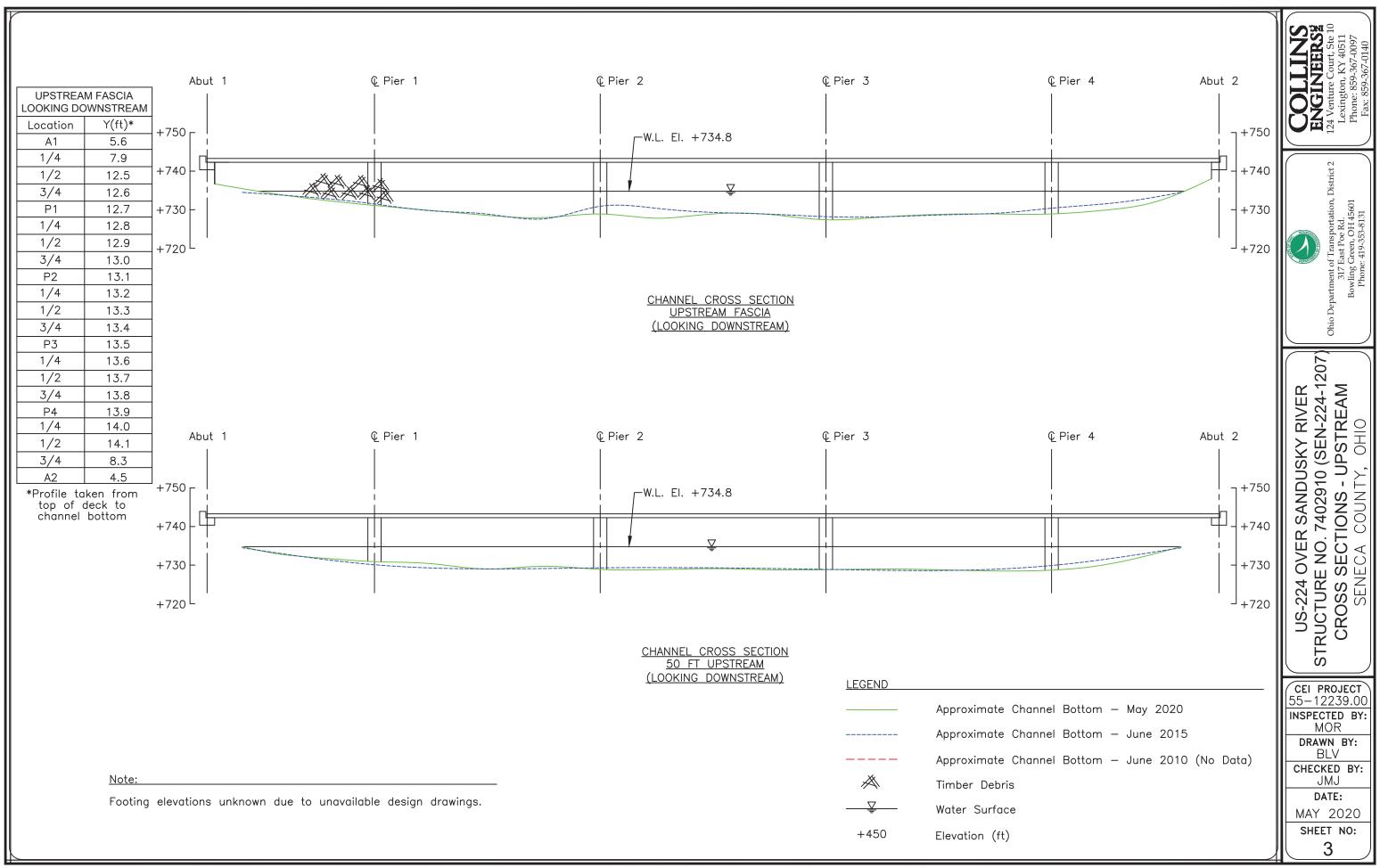


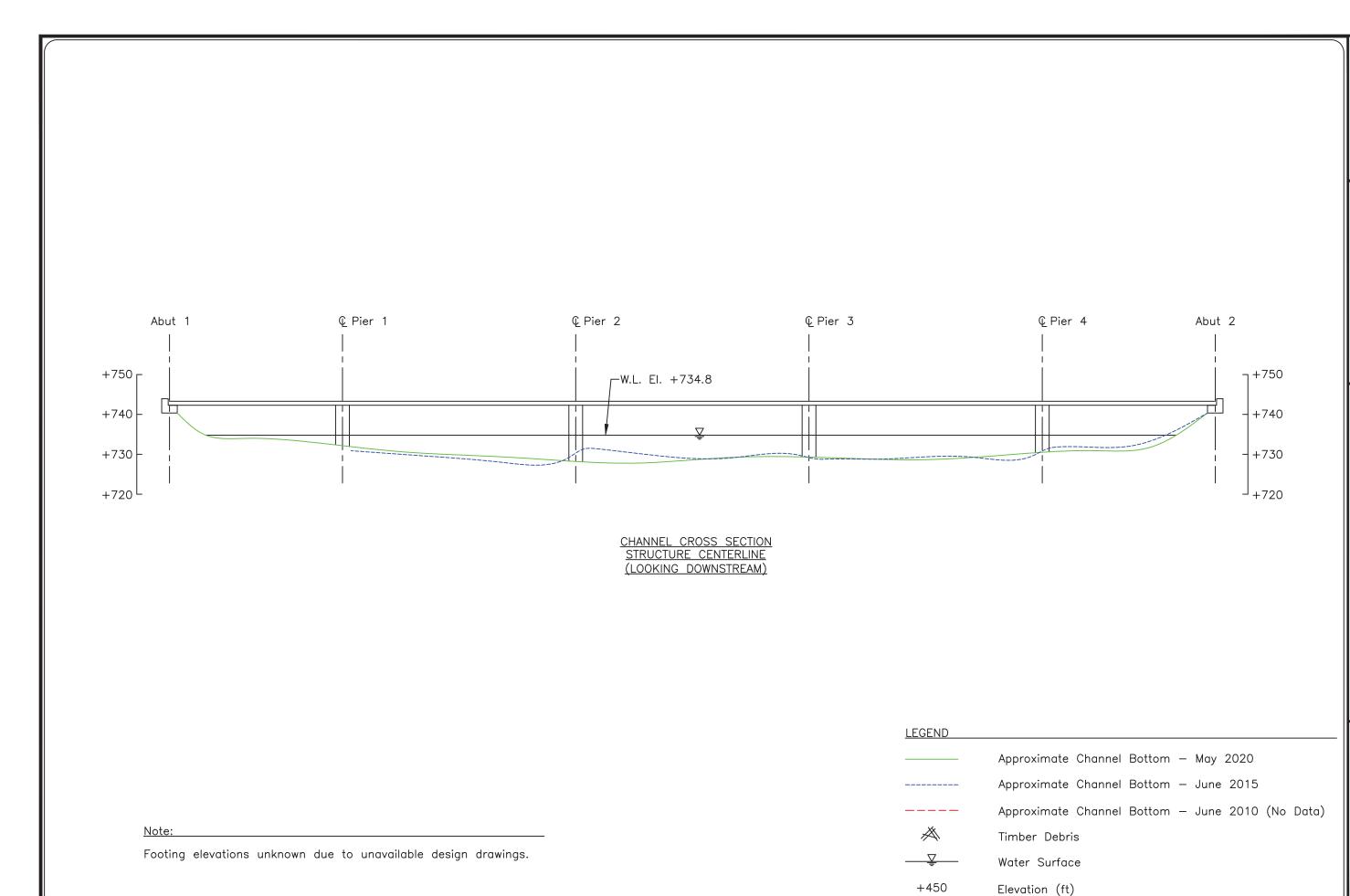
## EXHIBIT 1 – FIGURES







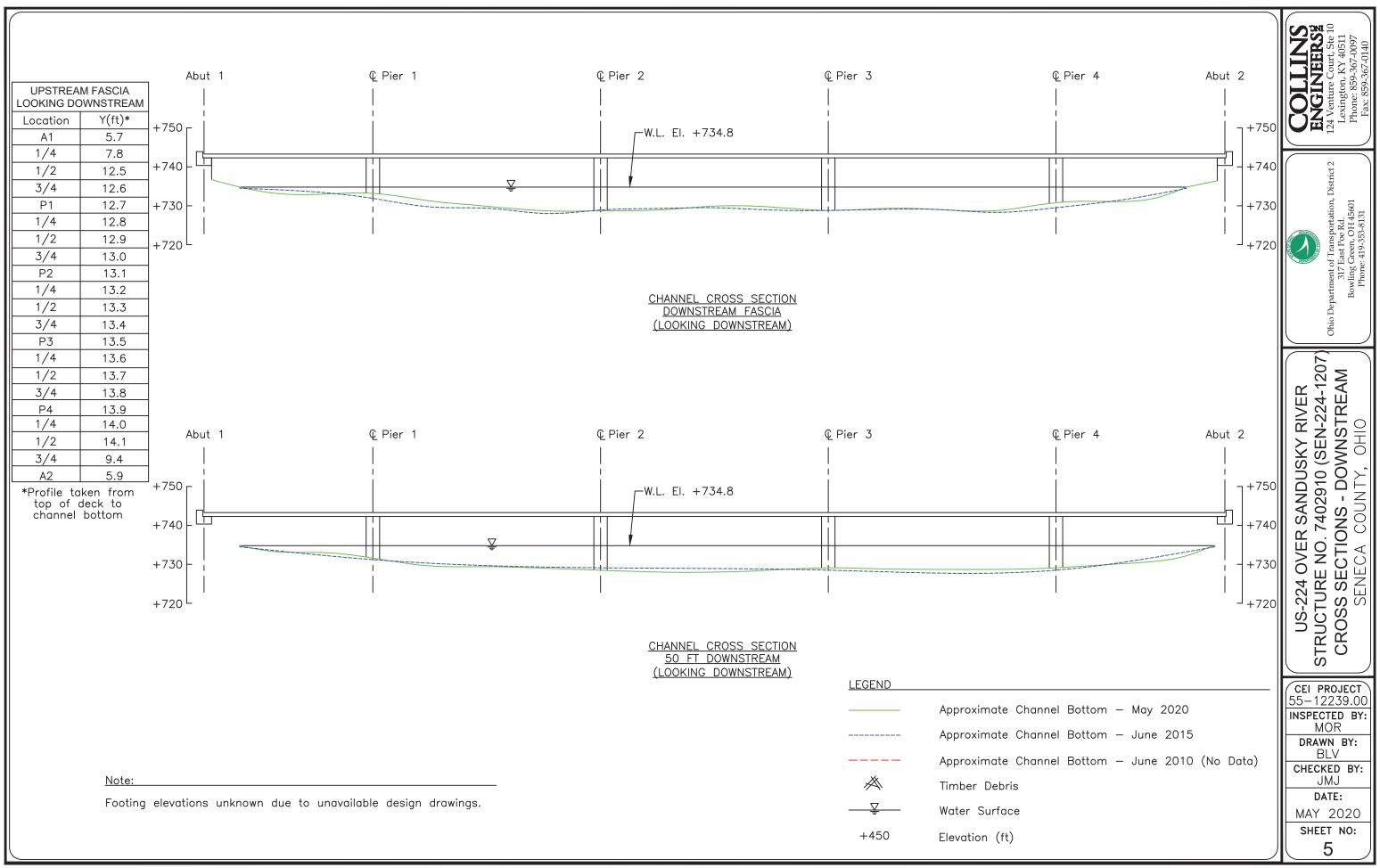


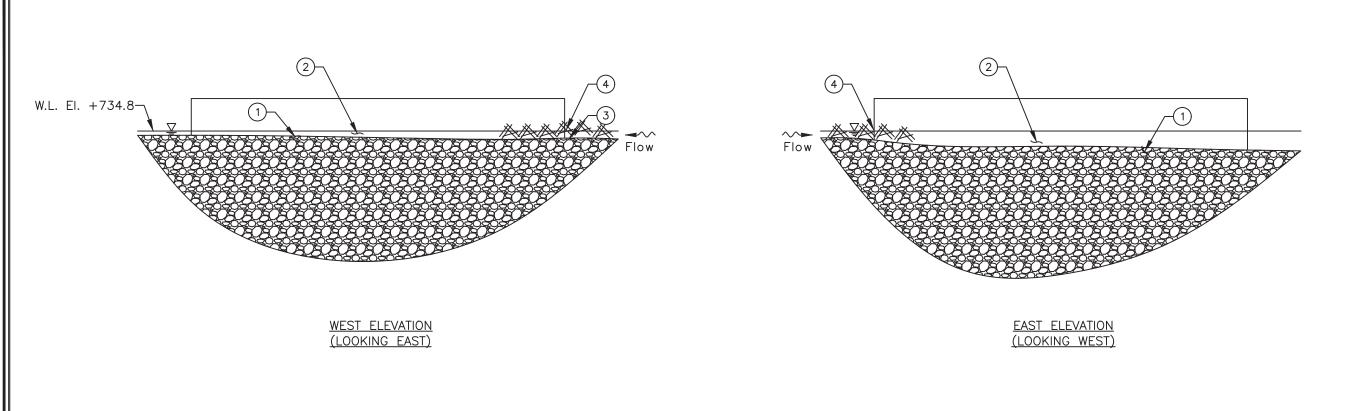


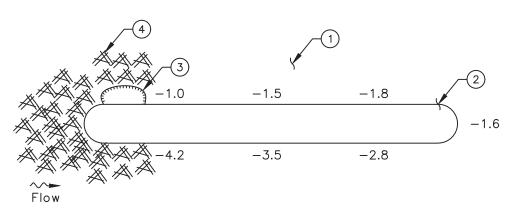
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**CEI PROJECT** 55-12239.00 INSPECTED BY: MOR DRAWN BY: BLV CHECKED BY: JMJ DATE: MAY 2020

SHEET NO:







<u>PLAN</u>

#### INSPECTION NOTES:

- 1 The channel bottom material consisted of timber debris with silt overlay with approximately 6 in. probe rod penetration.
- (2) The submerged portions of the pier were sound and smooth with no defects observed.
- 3 A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the southwest corner of the pier just downstream of the timber debris.
- Severe timber debris consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to 15 ft radius around the upstream ose from channel bottom to 4 ft above the waterline.

#### <u>LEGEND</u>

-2.7 Sounding Depth from Waterline (ft)

Approximate Channel Bottom — May 2020

A Timber Debris

<del>- ▼</del> Water Surface

Scour Depression

US-224 OVER S STRUCTURE NO. 74

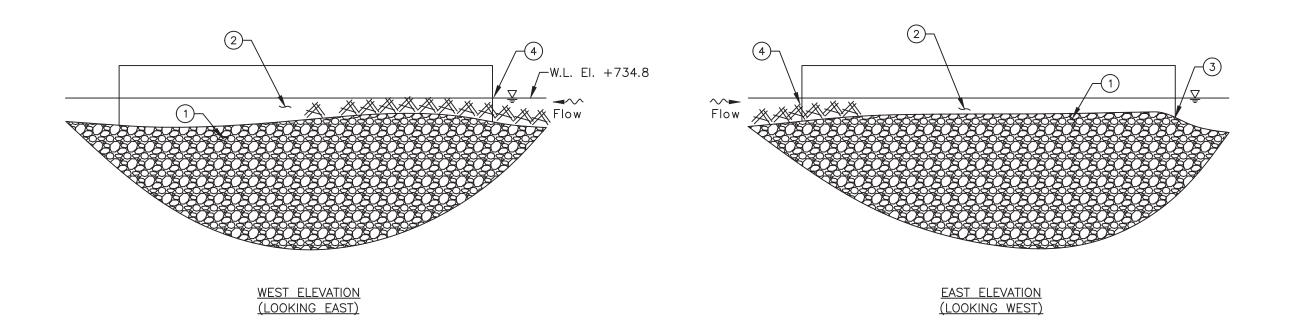
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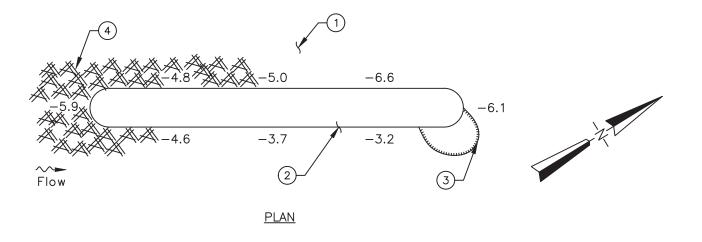
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JMJ

DATE:

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#### **INSPECTION NOTES:**

- (1) The channel bottom material consisted of timber debris with silt overlay with approximately 6 in. probe rod penetration.
- 2 The submerged portions of the pier were sound and smooth with no defects observed.
- 3 A scour depression measuring approximately 10 ft diameter by 2 ft deep was observed on the northeast corner of the pier just downstream of the timber debris.
- Heavy timber debris consisting of logs up to 2 ft diameter were observed on the upstream nose extending up to 10 ft radius around the upstream nose and along the north face from channel bottom to 2 ft above the waterline.

## <u>LEGEND</u>

-2.7 Sounding Depth from Waterline (ft)

— Approximate Channel Bottom — May 2020

Timber Debris

<del>- ♀</del> Water Surface

Scour Depression

epartment of Transport 317 East Poe Rd Bowling Green, OH

STRUCTURE NO. 7402910 (SEN-224-PIER 2

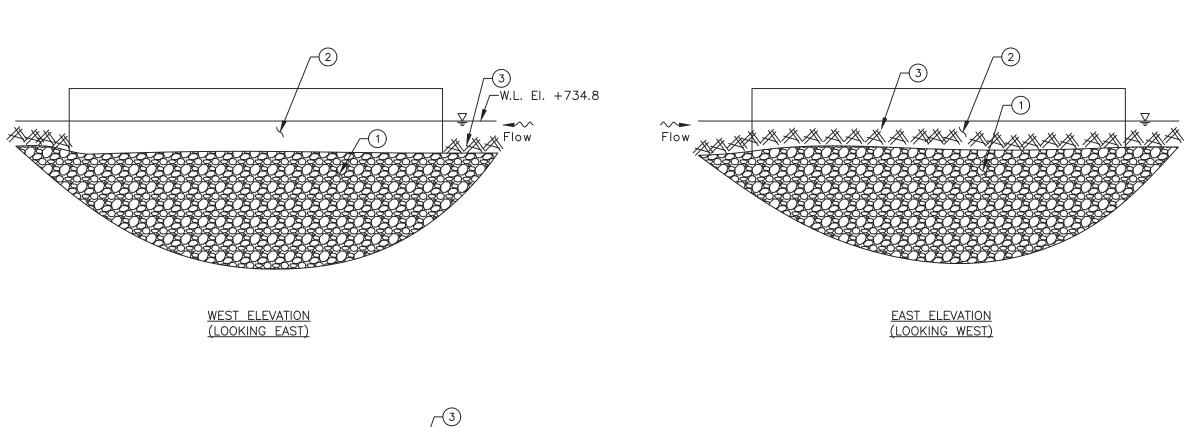
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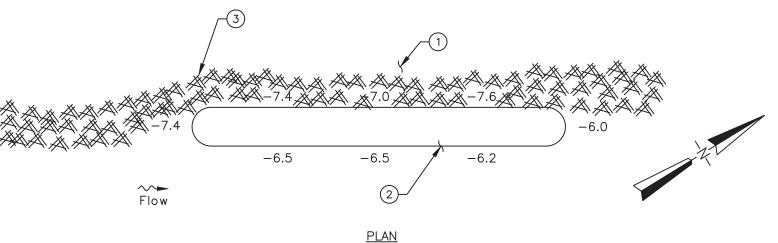
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DATE:

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#### **INSPECTION NOTES:**

- 1 The channel bottom material consisted of timber debris with silt overlay with approximately 6 in. probe rod penetration.
- (2) The submerged portions of the pier were sound and smooth with no defects observed.
- Moderate timber debris consisting of logs up to 1 ft diameter were observed on the upstream nose extending along the north face from channel bottom to 2 ft above the waterline.

#### **LEGEND**

-2.7 Sounding Depth from Waterline (ft)

— Approximate Channel Bottom — May 2020

X Timber Debris

<del>- ♀</del> Water Surface

Scour Depression

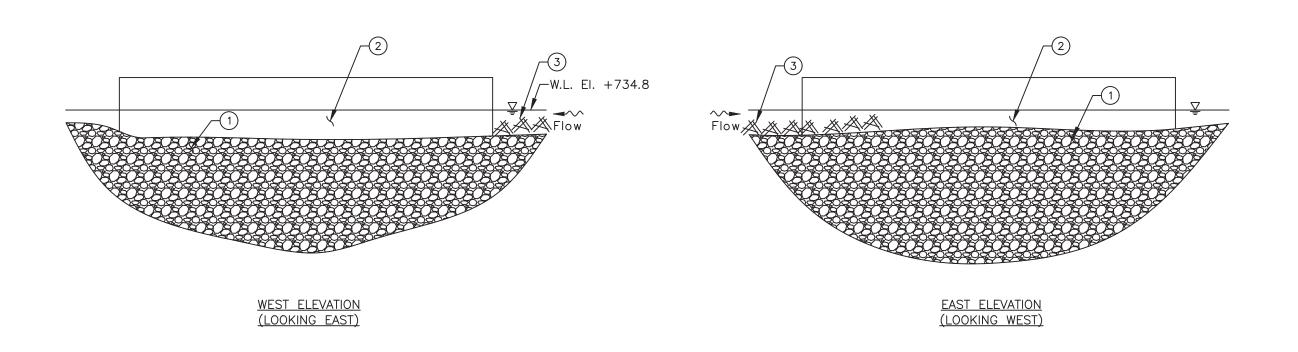
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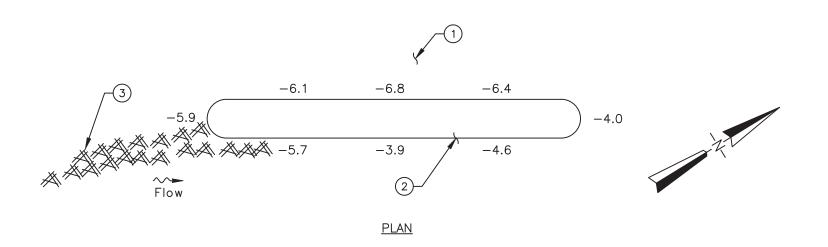
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#### **INSPECTION NOTES:**

- 1 The channel bottom material consisted of timber debris with silt overlay with approximately 6 in. probe rod penetration.
- (2) The submerged portions of the pier were sound and smooth with no defects observed.
- 3 Light timber debris consisting of branches up to 6 in. diameter were observed on the upstream nose extending up to a 5 ft radius from channel bottom to the waterline.

#### **LEGEND**

- -2.7 Sounding Depth from Waterline (ft)
- ——— Approximate Channel Bottom May 2020
- A Timber Debris
- <del>▼</del> Water Surface
  - Scour Depression

ENGINEER

124 Venture Court, Ste

nent of Transportation, Distr 317 East Poe Rd. wling Green, OH 45601 Phone: 419-353-8131

T N 224-1207)

US-224 OVER SANDUSKY I STRUCTURE NO. 7402910 (SEN PIER 4

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BLV

CHECKED BY:

JMJ

DATE:

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## EXHIBIT 2 – INSPECTION PHOTOGRAPHS







Photograph No. 1: Overall View of Structure No. 7402910 (SEN-224-1207), Looking North.



Photograph No. 2: Overall View of Structure No. 7402910 (SEN-224-1207), Looking South.







Photograph No. 3: View of the West Embankment Upstream of the Structure, Looking Southwest.



Photograph No. 4: View of the West Embankment at the Structure, Looking West.







Photograph No. 5: View of the West Embankment Downstream of the Structure, Looking Northwest.



Photograph No. 6: View of the East Embankment Upstream of the Structure, Looking Southeast.







Photograph No. 7: View of the East Embankment at the Structure, Looking East.



Photograph No. 8: View of the East Embankment Downstream of the Structure, Looking Northeast.







Photograph No. 9: View of the East Face of Pier 1, Looking Southwest.



Photograph No. 10: View of the West Face of Pier 1, Looking Northeast.







Photograph No. 11: View of the West Face of Pier 2, Looking Southwest.



Photograph No. 12: View of the East Face of Pier 2, Looking Northeast.







Photograph No. 13: View of the East Face of Pier 3, Looking Southwest.



Photograph No. 14: View of the West Face of Pier 3, Looking Northeast.







Photograph No. 15: View of the East Face of Pier 4, Looking Southwest.



Photograph No. 16: View of the West Face of Pier 4, Looking Northeast.







Photograph No. 17: View of the Typical Concrete Condition at the Waterline, Looking West.



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## EXHIBIT 3 – UNDERWATER DIVE INSPECTION PROCEDURE CHECKLIST



#### **Underwater Dive Inspection Procedure Checklist**

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a standalone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

### I. Bridge Identification

Agency with Inspection Responsibility: ODOT DISTRICT 2					
Dive Frequency:					
SFN: 7402910	_ Bridge Number (County-Route-SLM-SD): _SEN-224-1207				
Superstructure Type	Main Span Type: <u>STEEL BEAM</u>				
	Approach Span:REINFORCED CONCRETE				
Substructure Type	Abutment Type: REINFORCED CONCRETE				
	Pier Type:REINFORCED CONCRETE				
	Total Pier Count:4				
	Total Pier Count in water:4_				
	Foundations:UNKNOWN				
Feature Intersected	SANDUSKY RIVER				

## b. Photographs

#### Endview



Elevation



Underside

#### II. Office and Field Assessment

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

a. Channel Conditions	b. Anticipated Water conditions which
Waterway features	may affect the inspection
Rapid stream flows,	Cold Water (Apprx. Temp)
X Significant debris accumulation	Black water
Constricted waterway openings	Rapid stream flows
Soft or unstable streambeds	Near military facility
Meandering channels	Tribal fishing
Other which may promote scour and	Water quality
undermining of substructure elements	History of Log jams
Navigable Waterway	
Flow Controls	c. Identify factors that may accelerate the
	deterioration of the bridge elements:
	Highly corrosive water
	Unprotected steel members
	Other
Risk Factor Narrative:	

## III. Contacts Prior to Work

District 2 Bridge Engineer: David Geckle, P.E.					
Email: david.geckle@dot.ohio.gov – Phone: 419-373-4377					
Point of contact for immediate action such as closing the bridge due to findings					
Contact Bridge Owner14 (number) days before the proposed underwater inspection.					
Special contracting and scheduling procedures prior to inspection, include recommended lead time					

Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard			
Property Owner			
Access Equipment			
Lake or River draw- down			
Canal dry time			
Tree removal			
Othor			
Other:			
Other:			
Other.			

## IV. <u>Dive Team Shall Include the Following:</u>

Dive Team Narrative:					
The dive team consisted of one	e Team Leader (NBIS, P.	E., ADCI) and two Team Members (NBIS, UW, ADCI)			
-	ke notes, one diver, and	a three-member dive team: one supervisor to d one tender/standby diver. There shall be one			
v. <u>Site information</u>					
Navigable waterway:	Y / <u>N</u>	Anticipated current <u>1</u> ft			
If Yes, waterway river point	<u>N/A</u>	Scour Critical (item 113):8			
Anticipated water visibility depth	h <u>&lt;1</u> ft	POA in place: Y/N			
Anticipated Dive depth	8ft	Scour Monitoring devices present: Y/N			
Verify the Scope of Services whe	n work is contracted fo	r the procedure for underwater elements that			
are not in water during an inspec	ction.				
Site Information Narrative:					

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with
		Commentary
Piers and Number of	4	100% LEVEL I
Columns		10% LEVEL II
Abutment	N/A	
Culvert	N/A	
Scour Countermeasures	N/A	
Fenders or Dolphins	N/A	

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

## a. The inspection should be conducted The note taker should work alongside the dive using: team. Chest waders Hip waders d. Access to the waterway should be \_\_X\_\_Diving equipment obtained from the shore (north bank, SCUBA (Note that ADCI Consensus southwest quadrant, driveway 30 yards Standards require communication systems be employed for both SCUBA and north etc.) Surface-Supplied (whether air or mixedgas) dive modes) WEST SHORELINE (DAYS IN PARKING LOT) SCUBA with communication e. The maximum depth of the channel is X Surface Supplied with typically measured feet from communication AT THE UPSTREAM NOSE OF PIER 3 b. The channel bottom should be sounded utilizing X Digital fathometer Reference Datum: Top of Cap at the \_\_X\_\_Telescoping survey rod Downstream Nose of Pier 2 \_\_\_\_ acoustic imaging Soundings should be dictated by the scope of work. When not detailed in the scope they c. During the inspection, the divers should should be repeated from the previous work from soundings. If neither exist then they need to be Shore taken in a grid pattern between substructure X Boat units 100' upstream and 100' downstream. Either

VI.

**Equipment and Field Logistics** 

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Created: COLLINS ENGINEERS, INC.	Date:	9/25/2020	
Updated By:	Date:		
VIII. Other Narrative Not Included In Prev			

VII.

**Inspection Procedure History**