

GENERAL NOTES



metric units

Item 516 Integral Abutment Expansion Joint Seal, As Per Plan

Install a 900 mm wide strip, 2.5 mm thick, general purpose, heavy duty neoprene sheet with nylon fabric reinforcement at locations shown in the plans. Secure the 900 mm wide neoprene sheeting to the concrete with 32 x 3 mm (Length x Shank Diameter) galvanized button head spike through a 25 mm outside diameter, 3 mm galvanized washer. Maximum fastener spacing is 225 mm. Other similar galvanized devices which will not damage either the neoprene or the concrete may be used subject to the approval of the Engineer.

Center the neoprene on all joints. For horizontal joints, secure the horizontal neoprene strip by using a single line of fasteners, starting at 150 mm (+/-) from the top of the neoprene strip. For the vertical joints secure the vertical neoprene strip by using a single vertical line of fasteners, starting 150 mm (+/-) from the vertical edge of the neoprene strip nearest to the centerline of the roadway. For vertical joints, install 2 additional fasteners at 150 mm center to center across the top of the neoprene strip on the same side of the vertical joint as the single vertical row of fasteners is located.

The vertical neoprene strips should completely overlap the horizontal strips. Laps in the length of the horizontal strips due to the material manufacturing shall be at least 300 mm in length, if not vulcanized or adhesived, or 150 mm in length if the lap is vulcanized or adhesived. No laps are acceptable in vertically installed neoprene strips.

The neoprene sheeting shall be 2.5 mm thick general purpose, heavy duty neoprene sheet with nylon fabric reinforcement. The sheeting shall be "Fairprene Number NN-0003", by E.I. Dupont De Nemours and Company Inc., "Wingprene" by Goodyear Tire and Rubber Company, or an approved alternate. The neoprene sheeting shall conform to the following:

<u>Description of test</u>	<u>ASTM Method</u>	<u>Requirement</u>
Thickness, mm	D751	2.5 ± 0.25
Break Strength, grab WXF, N, minimum	D751	3130 x 3130
Adhesive 25 mm strip, 50 mm Min, N minimum	D751	27
Burst strength (mullen) MPa, minimum	D751	9.65
Heat aging 70 hours T 100°C 80° bend without cracking	D2136	No Cracking of Coating
Low temp. brittleness 1 hour at 40°C, bend around 6 mm mandrel	D2136	No Cracking of Coating

Payment for labor, materials and installation of these items shall be included in Item 516 Integral Abutment Expansion Joint Seal, As Per Plan.

Utility Lines: All expense involved in relocation (installing) the affected utility lines shall be borne by the Utility(ies). The Contractor and Utility(ies) are to cooperate by arranging thier work in such a manner that inconvenience to either will be held to a minimum.

Item 503- Unclassified Excavation, as per plan: Unclassified excavation shall be in accordance with item 503 except that the backfill material behind the abutment shall be 304 granular material placed in lifts not to exceed a thickness of more than 150mm

Abutment Backfill: Abutment Backfill shall be limited to 300mm below the bridge seat elevation until the beams have been in place for at least 3 days.

Pile Design Loads (Ultimate Bearing Value): The Ultimate Bearing Value is 1029 kN per pile for the 350mm diameter cast-in-place reinforced concrete abutment piles. The Ultimate Bearing Value is 1131 kN per pile for the 400mm diameter cast-in-place reinforced concrete pier piles.

outment piles:
16 piles 12 meters long, estimated length
16 piles of order length 13.5 meters long
2 " "

per piles:
16 piles 17 meters long, estimated length
16 piles of order length 18.5 meters long

Item 507- 400mm Cast-In-Place Piles, as per plan (Pile Wall Thickness):
The responsibility of choosing and providing a satisfactory pile wall thickness for this project shall be borne by the Contractor except that the pile wall thickness shall not be less than 5.5mm. If a pile wall thickness greater than 5.5mm is necessary to resist the installation driving stress, the Contractor shall make this determination and shall furnish a pile with an acceptable wall thickness.

ESTIMATED QUANTITIES & GENERAL NOTES

Bridge No. OTT-2-10848
over Turkey Creek

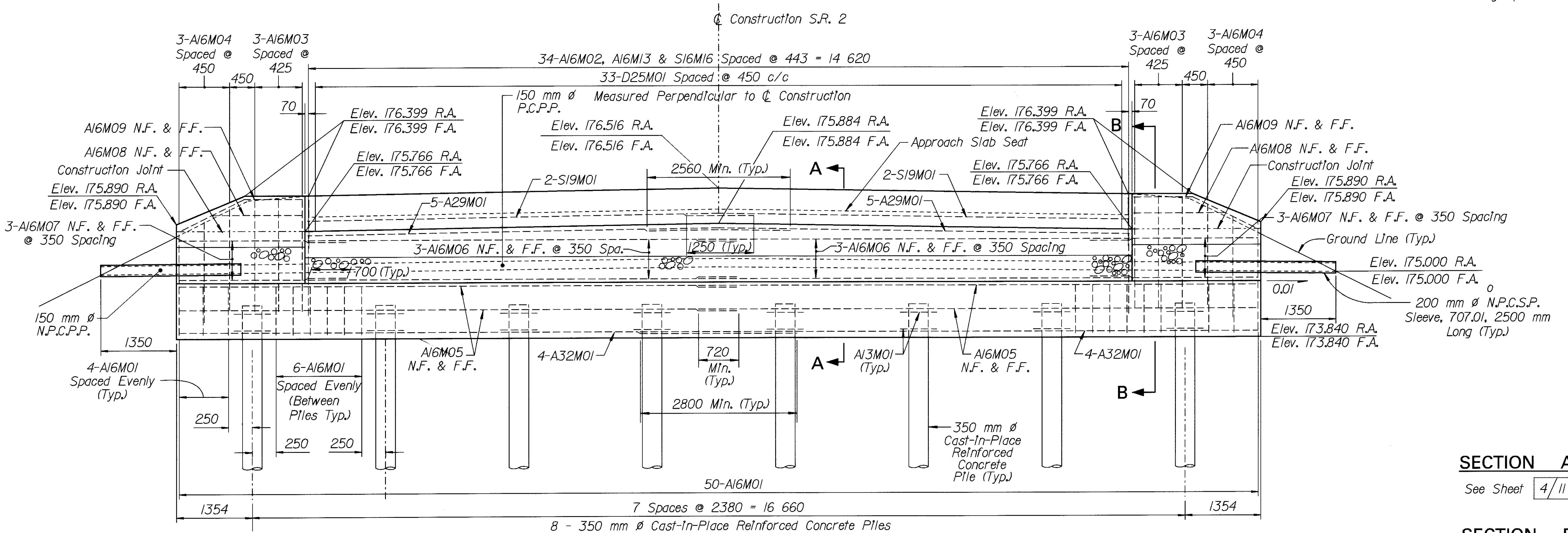
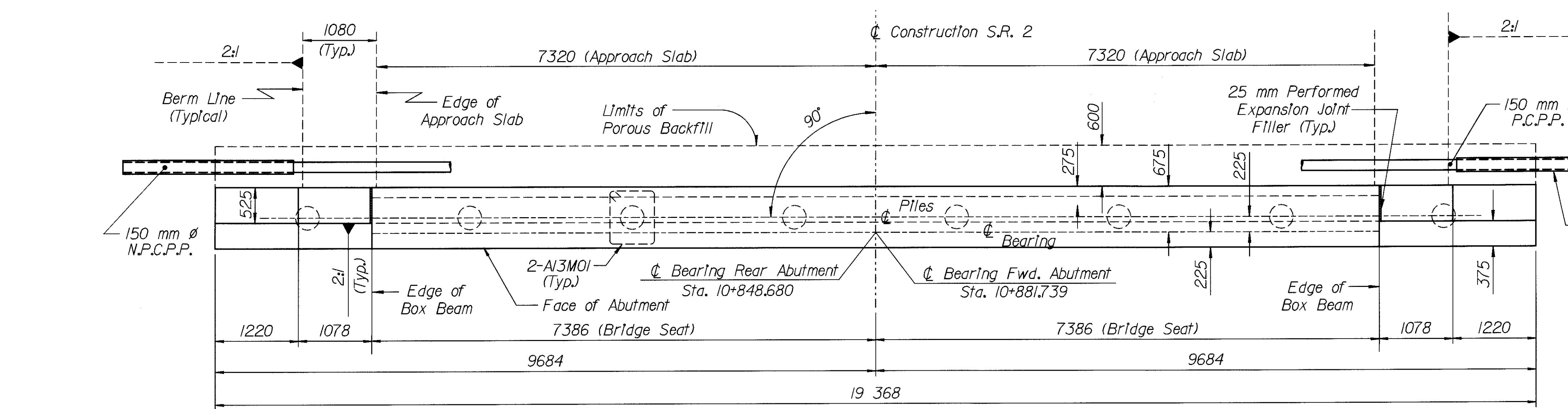
OTT-2-10.735/17.135

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DESIGNED	DRAWN <i>EJS</i>	REVIEWED <i>JRC</i>	DATE 5-5-9
CHECKED	REVISED <i>JTB</i>		STRUCTURE FILE NUMBER 6200109

LEGEND

N.F. = Near Face
R.F. = Rear Face
R.A. = Rear Abutment
F.A. = Forward Abutment
P.C.P.P. = Perforated Corrugated Plastic Pipe
N.P.C.P.P. = Non-Perforated Corrugated Plastic Pipe,
 Including Specials
N.P.C.S.P. = Non-Perforated Corrugated Steel Pipe,
 Including Special



NOTES

Porous Backfill: With filter fabric, 600mm thick shall extend up to the plane of the subgrade, to 300mm below the embankment surface, and laterally to the ends of the wingwalls. Geotextile fabric shall conform with 712.09, Type A. The bottom of the porous backfill shall be sloped (0.08 min.) laterally to drain. Geotextile fabric is included with porous backfill for payment.

ABUTMENT ELEVATION

Abutment Elevation
Abutment Concrete above the beam seat shall not be placed until the prestressed box beams have been placed.

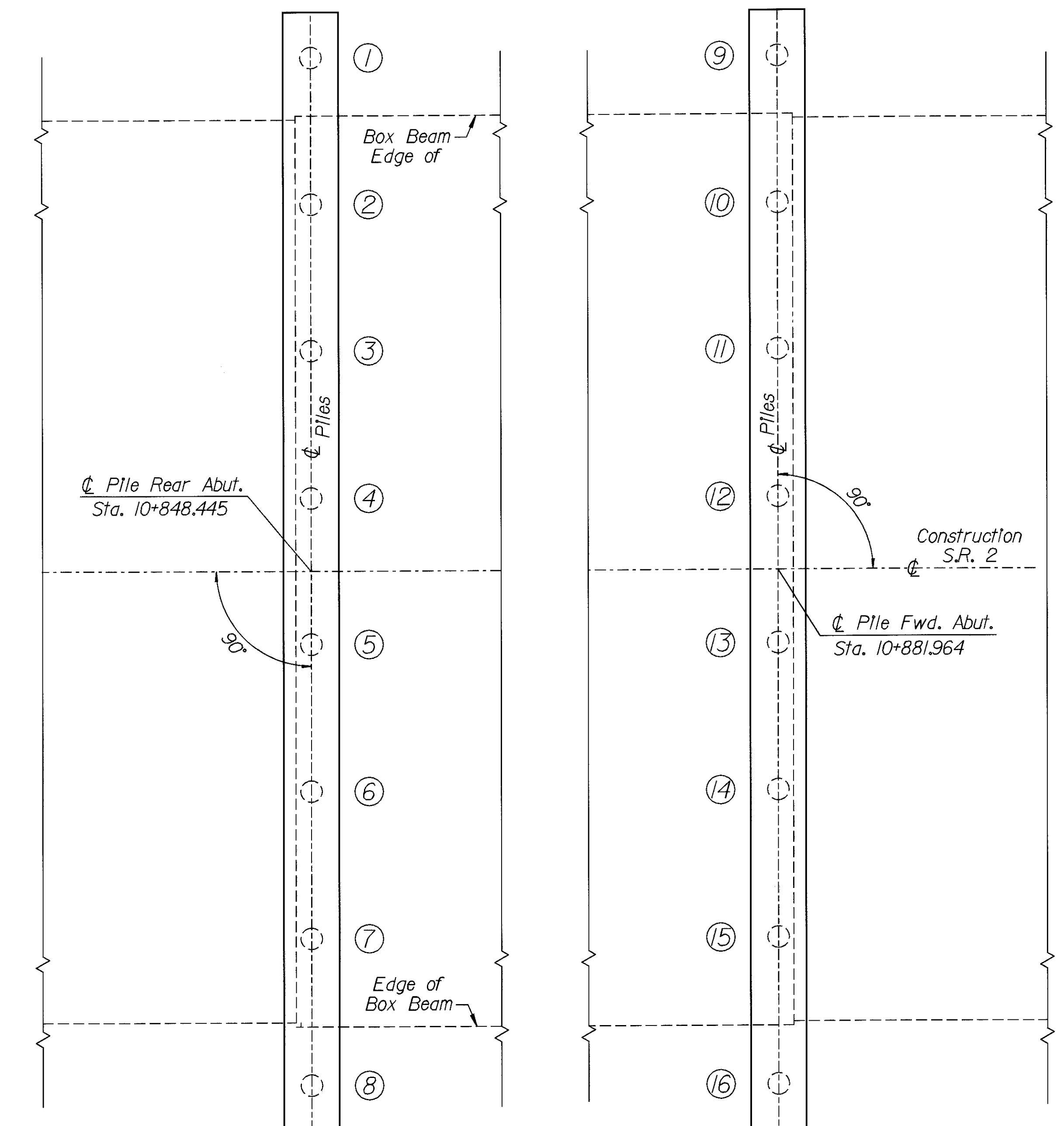
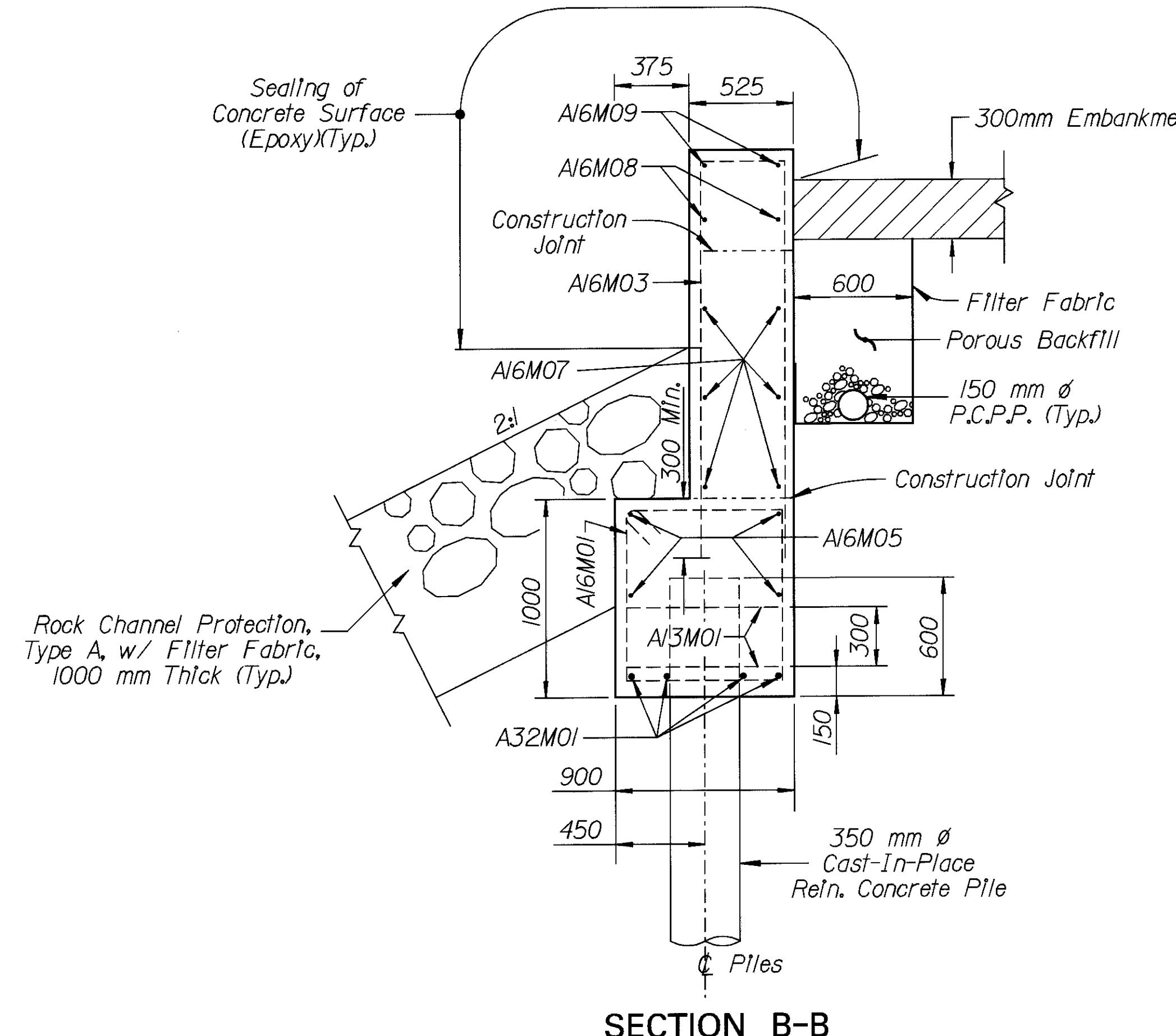
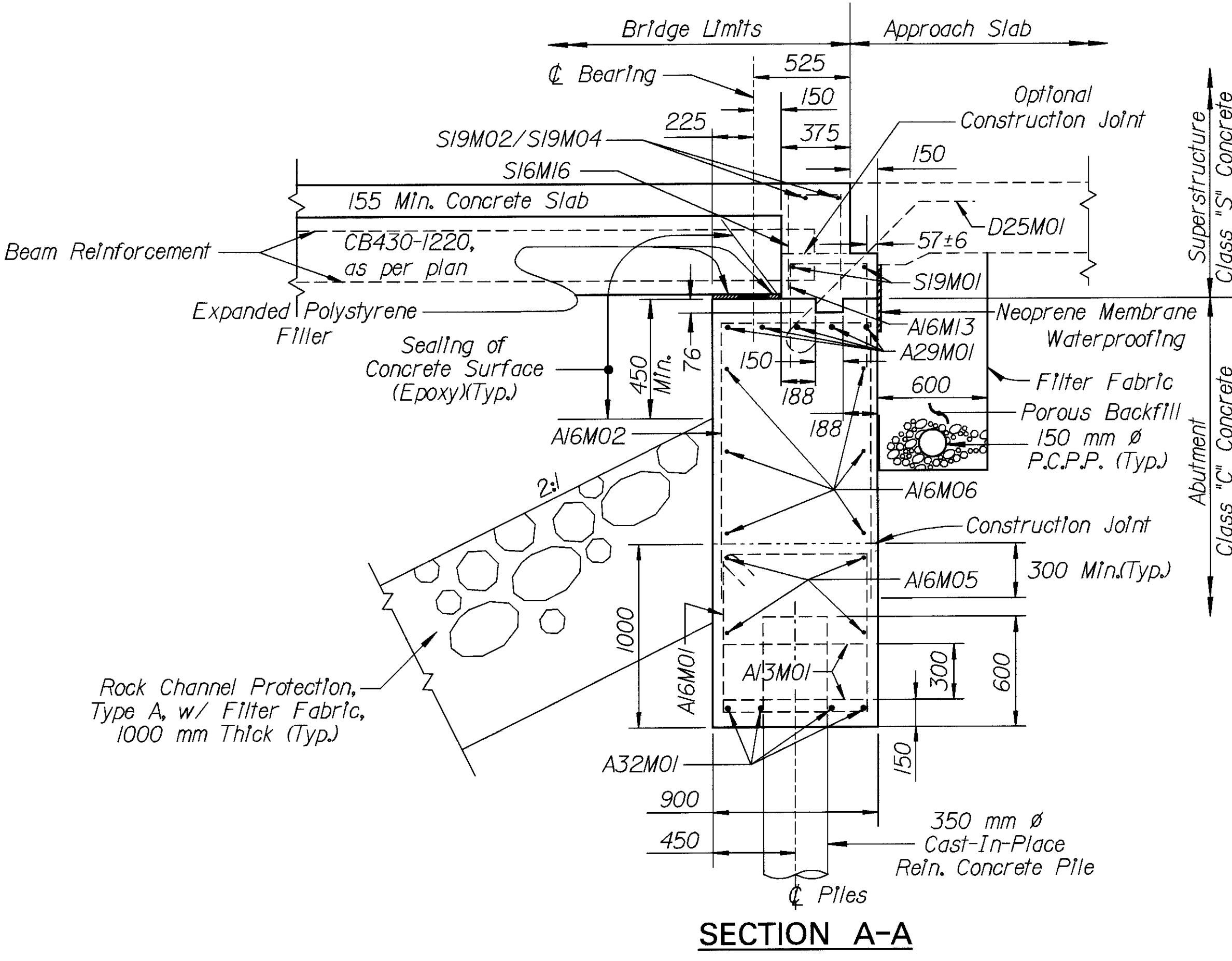
All dimensions are in millimeters, unless otherwise noted except for stations and elevations which are in meters.

SECTION A-A

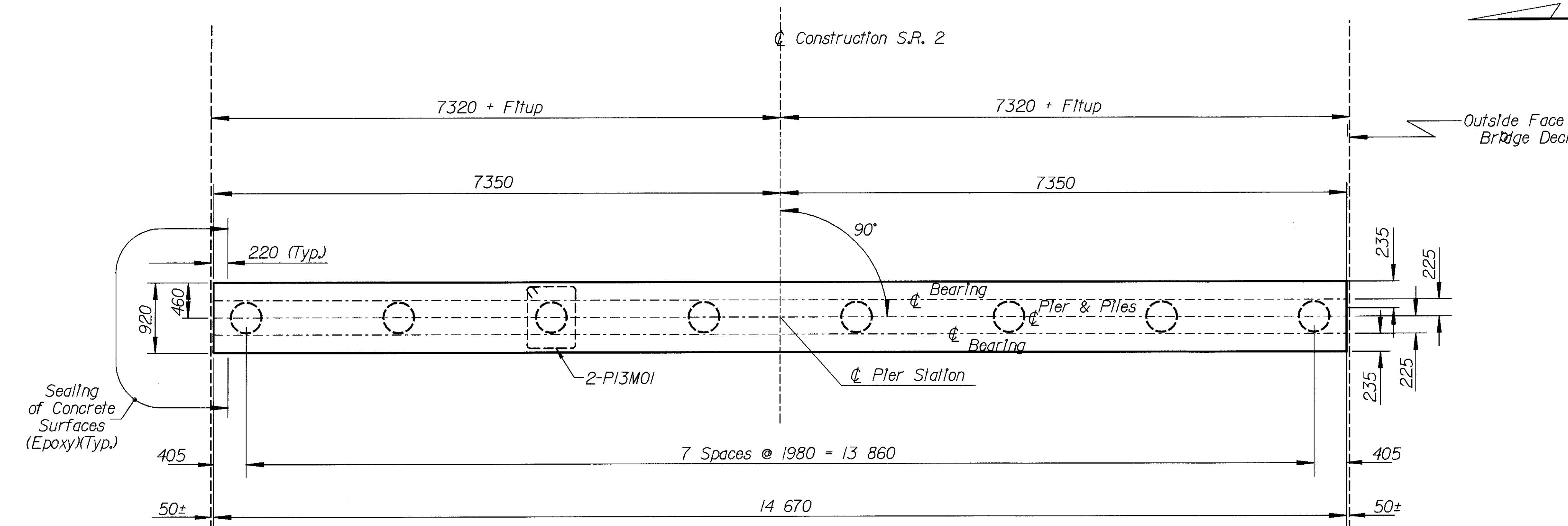
See Sheet 4 / 11

SECTION B-B

See Sheet 4 / 11

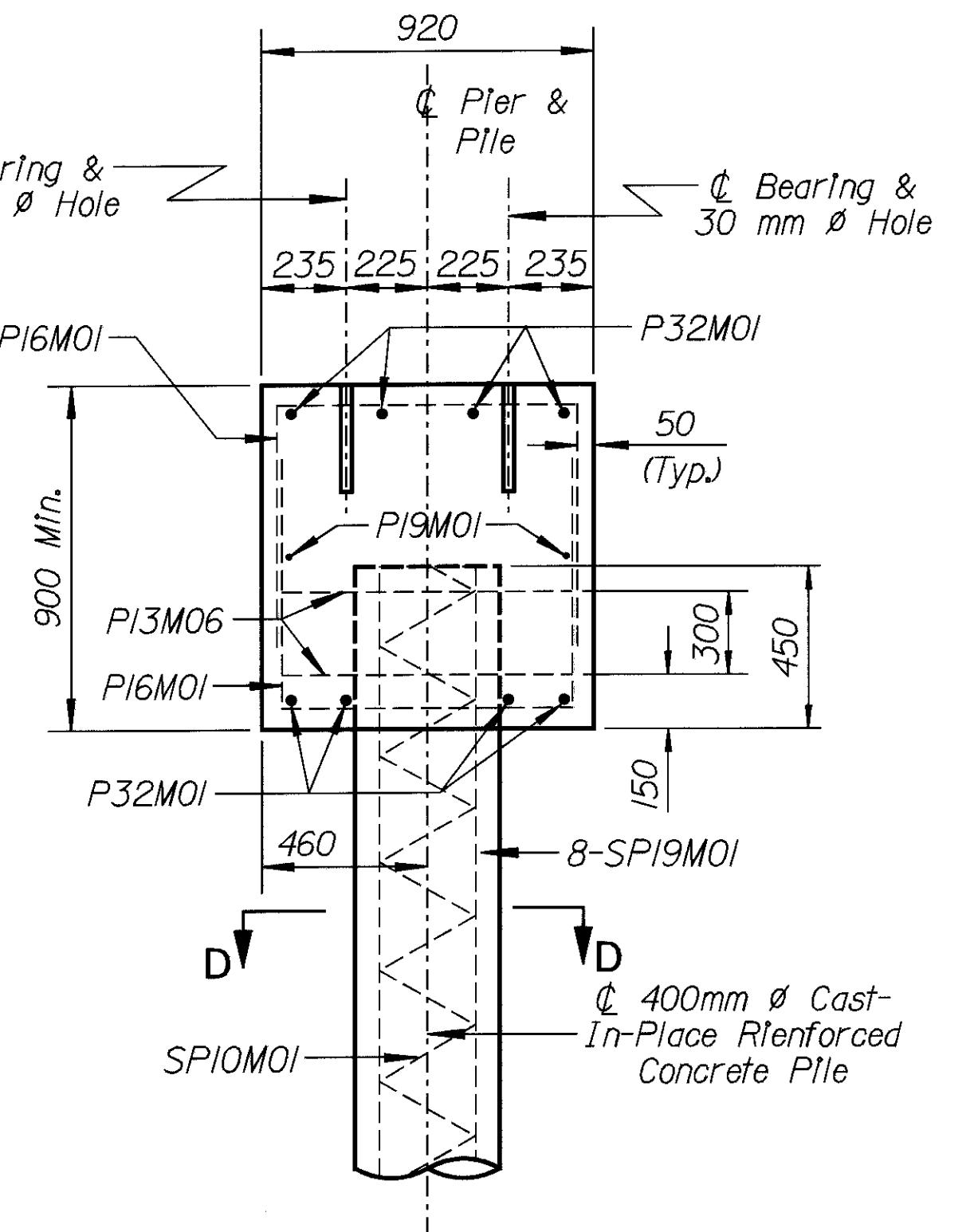
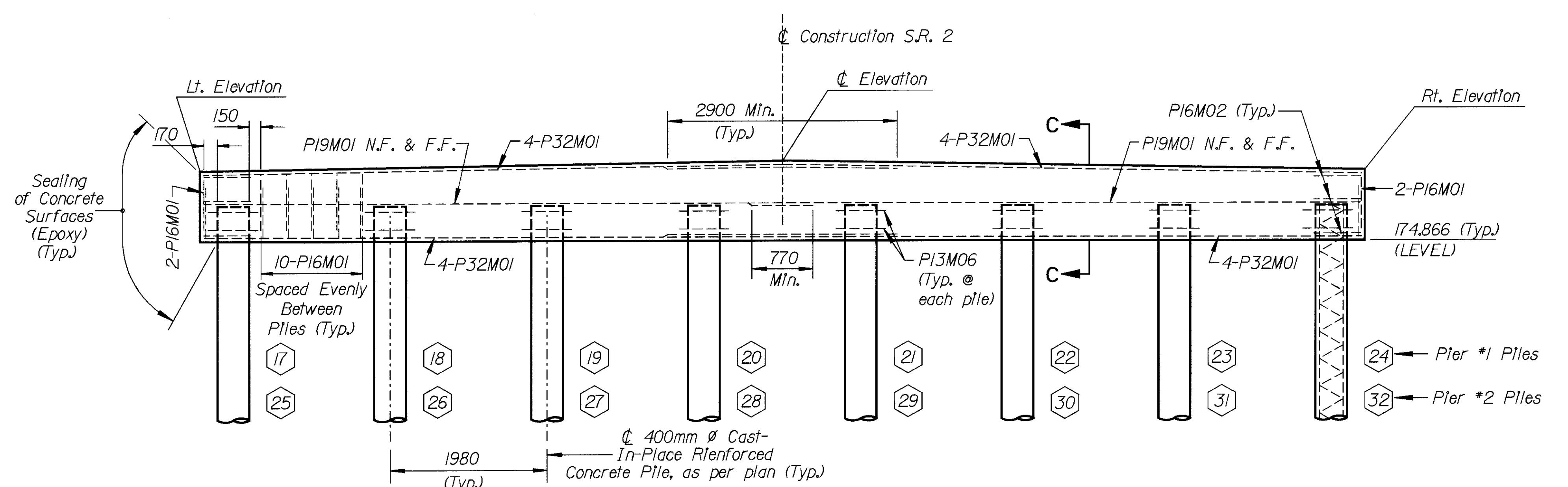

LEGEND

P.C.P.P. - Perforated Corrugated Plastic Pipe

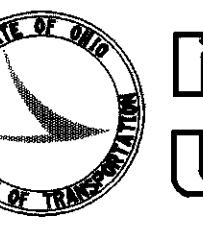

CENTERLINE PIER STATIONS AND ELEVATIONS

Pier Number	∅ Pier Station	Lt. Elevation	∅ Elevation	Rt. Elevation
Pier #1	Sta. 10+859.508	175.766	175.884	175.766
Pier #2	Sta. 10+870.911	175.766	175.884	175.766

NOTE: Elevations are along the ∅ of the Piers

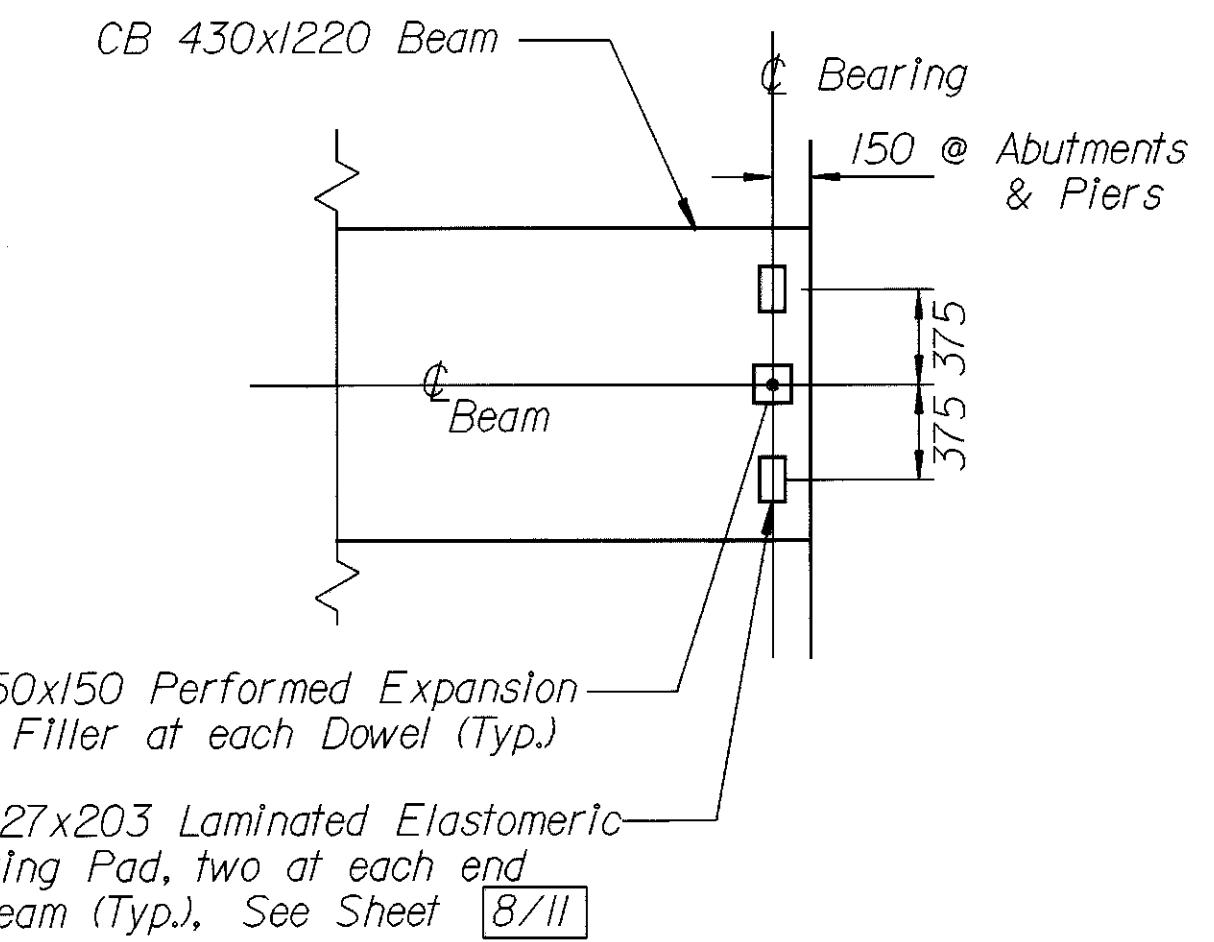
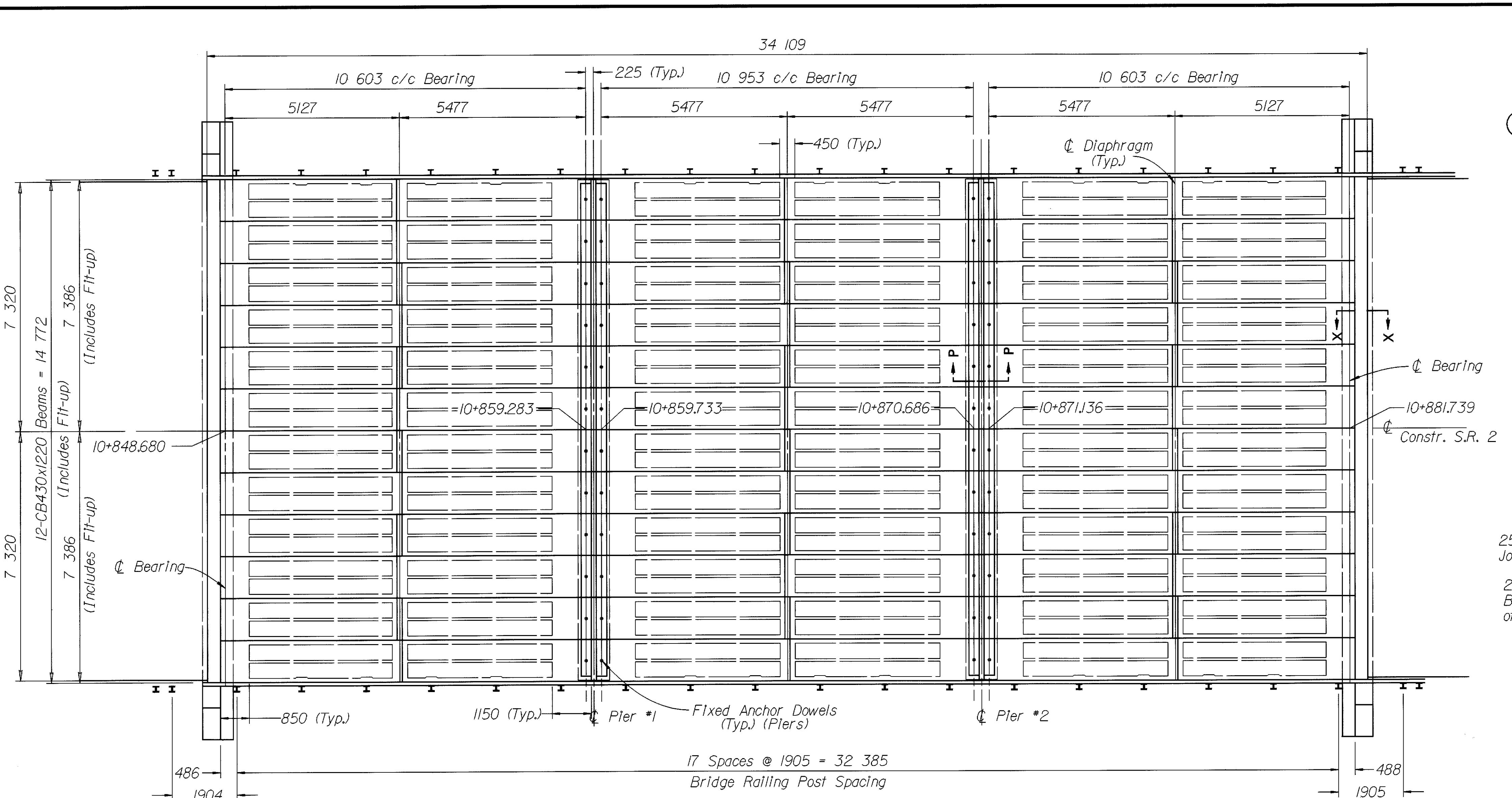

SECTION C-C
SECTION D-D

PIER ELEVATION VIEW

All dimensions are in millimeters, unless otherwise noted
except for Stations and Elevations which are in Meters.



**metric
units**

DESIGN AGENCY
DISTRICT ONE
PRODUCTION DEPARTMENT



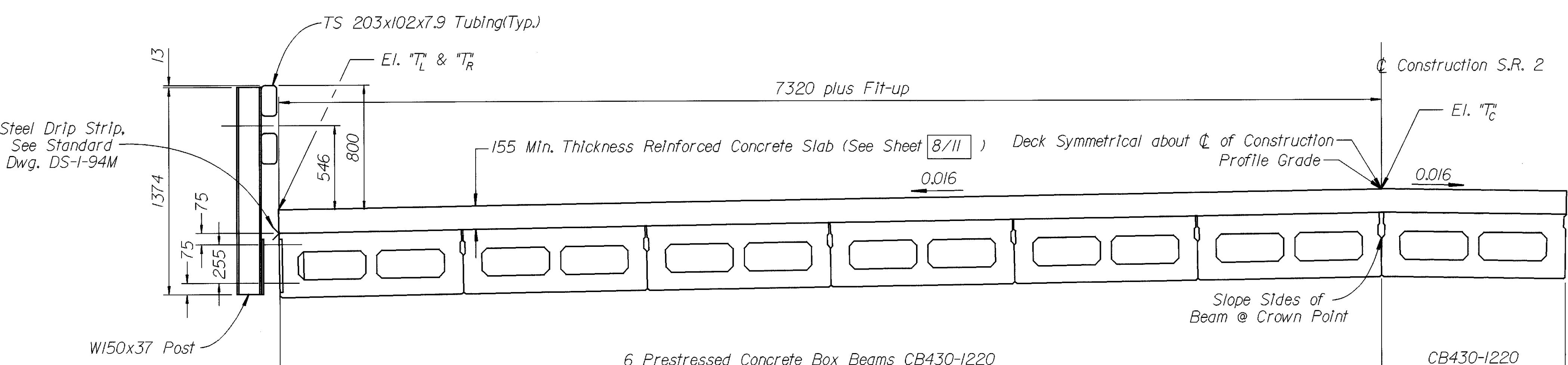
SUPERSTRUCTURE DETAILS
Bridge No. OTT-2-10848
over Turtle Creek

OTT-2-10.735/17.135

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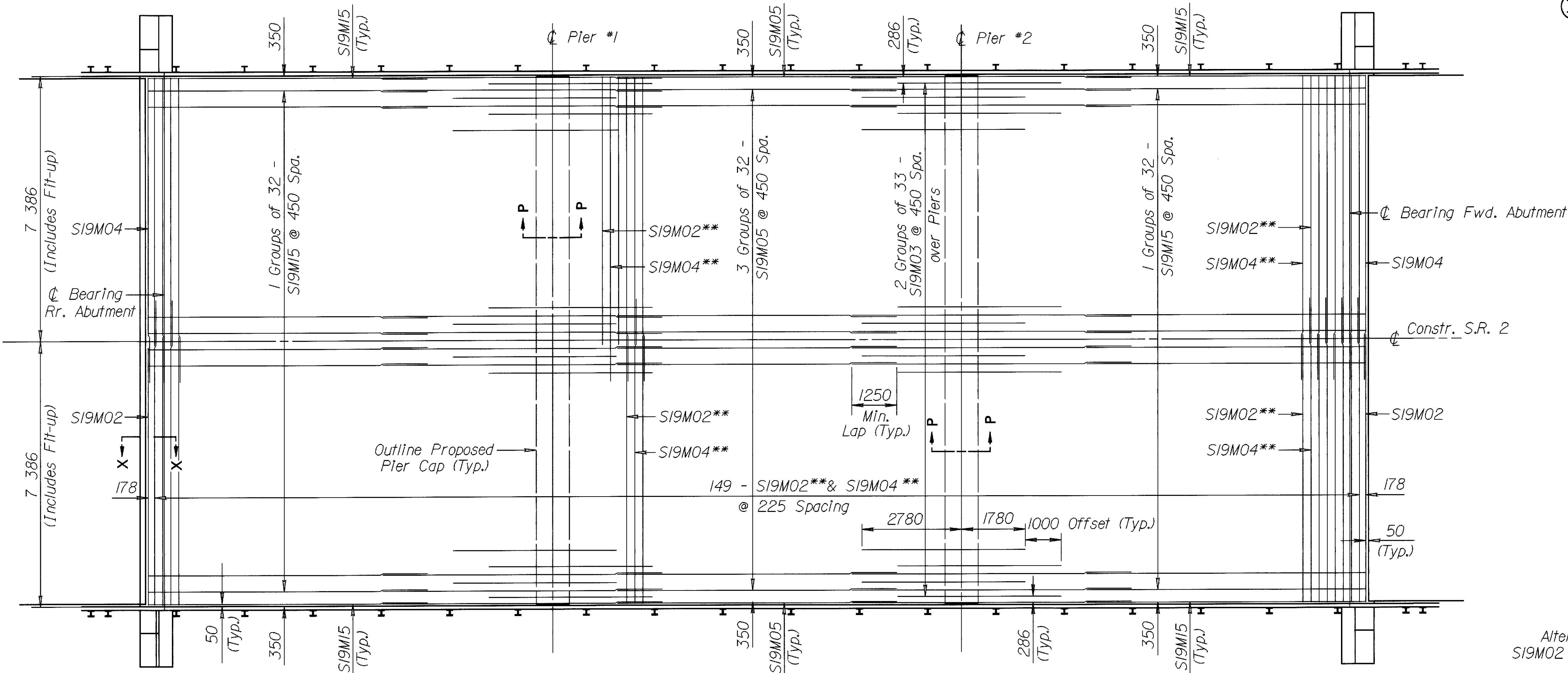
SECTION X-X
See Sheet 8/II

SECTION P-P
See Sheet 7/II



NOTE: All dimensions are in millimeters unless otherwise noted.
Stations and elevations however are in meters.

NOTE: All dimensions are in millimeters unless otherwise noted.
Stations and elevations however are in meters.


HALF TRANSVERSE SECTION

* Include with Item 865
Prestressed Concrete Beam

** Alternate Bars so Laps
Alternate from Right to Left of $\frac{1}{2}$ Construction

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REF.FILE *3: \$\$\$\$\$\$REF FILENAME3\$\$\$\$\$
REF.FILE *3 LEVELS ON: \$\$\$\$\$\$REF FILE3\$\$\$\$\$
REF.FILE *4: \$\$\$\$\$\$REF FILENAME4\$\$\$\$\$

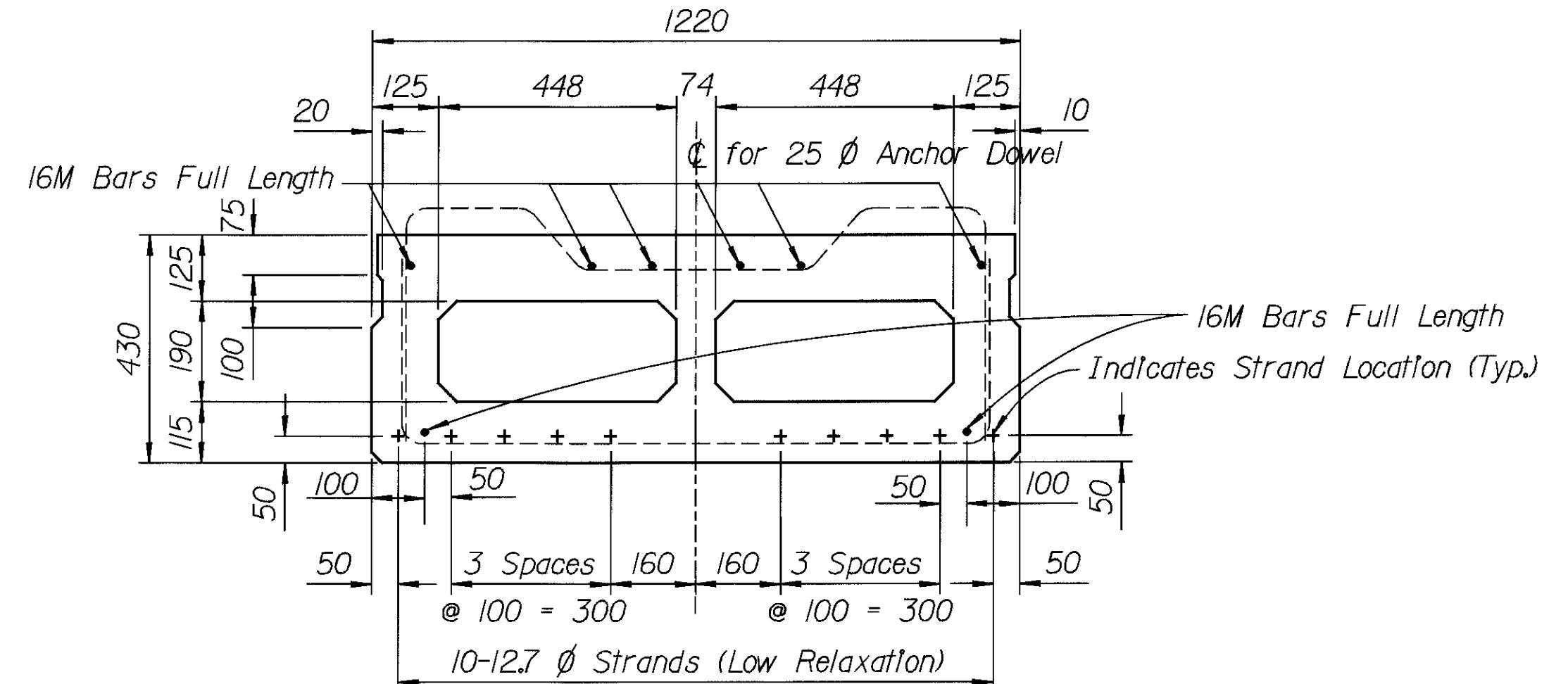
Cambridge

Camber
Calculated camber at time of paving, including allowance for camber growth due to creep, is 27mm.

Calculated deflection due to weight of Reinforced Concrete Slab and railing is 5mm.

Net final camber of beams is 22mm. This is 22mm in excess of the amount required to place the top of the beam parallel to profile grade. This excess amount shall be compensated for by thickening the Reinforced Concrete Slab from 155mm at center of span to 177mm at the ends.

Elastomeric Bearings shall comply with Item 5/6 and Articles 18.2.5 through 18.2.8 of Section 18, Bearing Devices, Division II, Construction of the AASHTO Standard Specification for Highway Bridges. Bearings shall be Grade 3, 50 durometer elastomer, and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the unit price bid for bearings, each

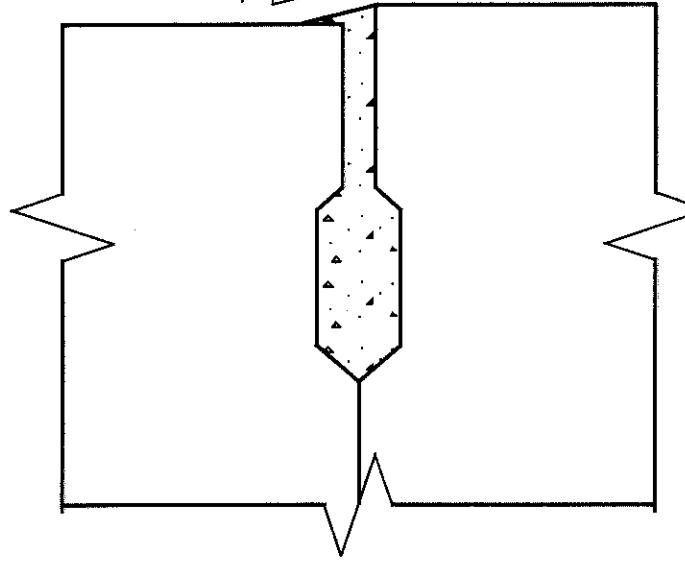


CB430-1220, AS PER PLAN

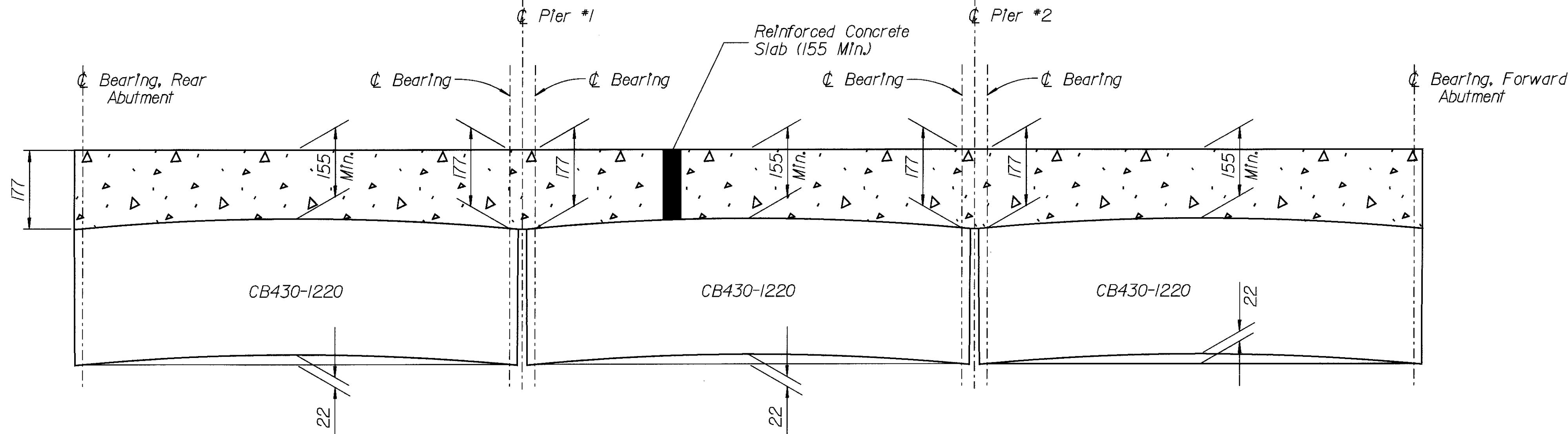
For Other Beam Detail Information See Std. Dwg. PSBD-I-93M

Note: All dimensions are in millimeters unless otherwise noted.

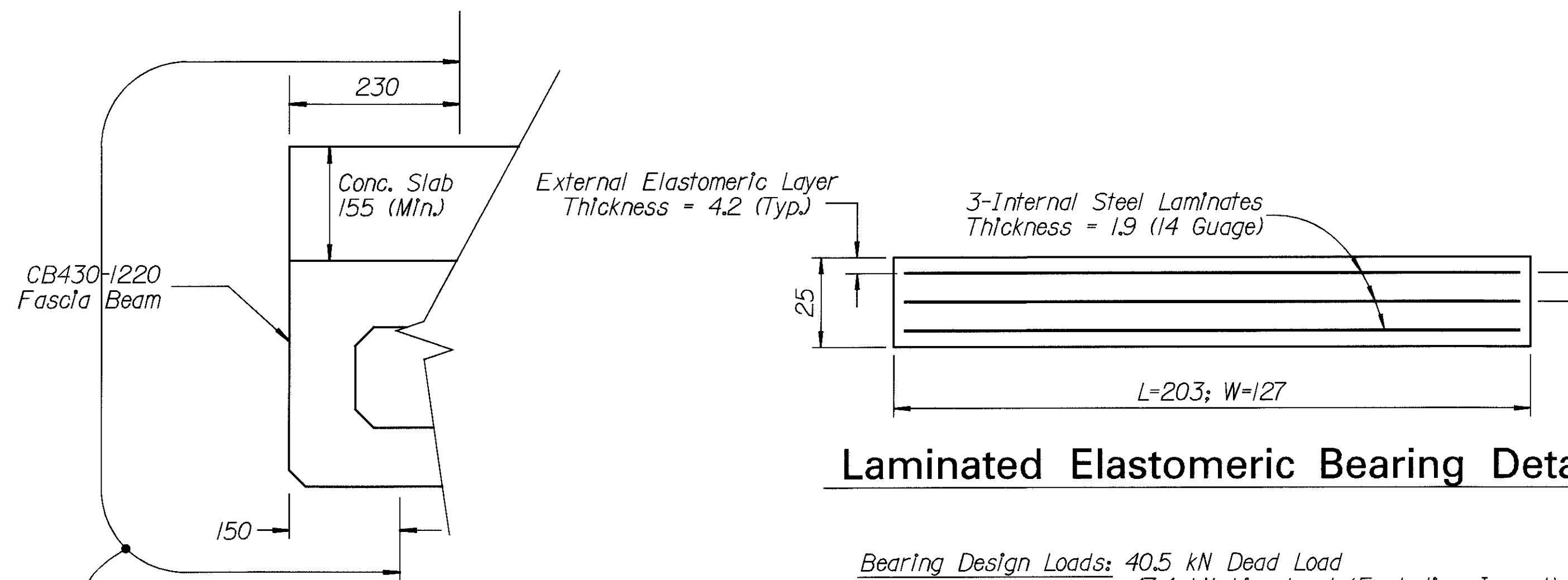
Maximum Slope



Detail of shear key mortor where adjacent beams have vertical offset due to different camber, skew effect.



Deck Thickness Diagram

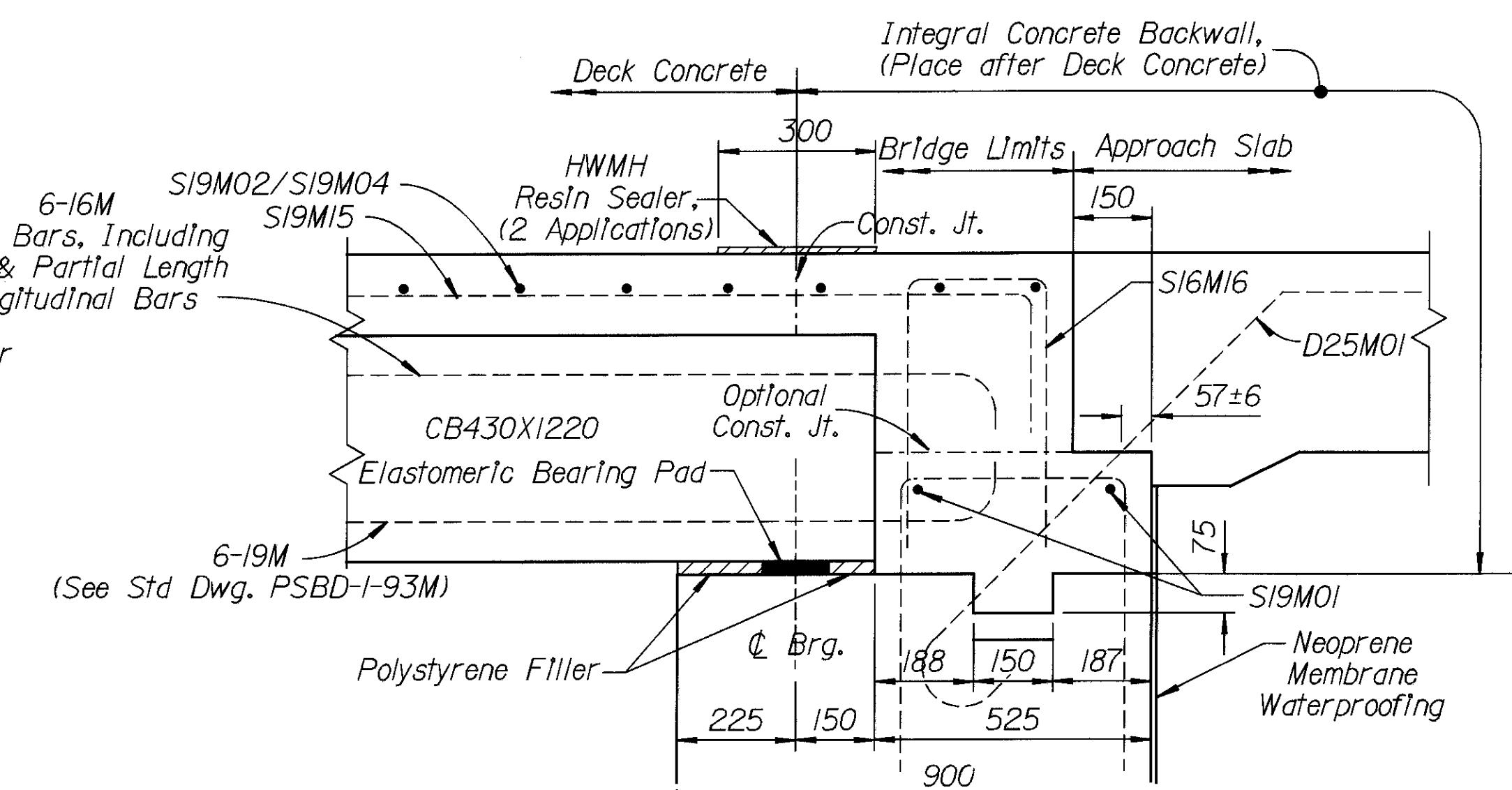


Laminated Elastomeric Bearing Details

Bearing Design Loads: 40.5 kN Dead Load
47.4 kN Live Load (Excluding Impact)
Maximum Design Load = 87.9 kN

DECK SCREED ELEVATIONS(*)				
Location	¢ Brg. R.A.	½ Span 1 thru 3	¢ Pier 1 & 2	¢ Brg. F.A.
Edge of bridge Deck El. "Tr & Tl"	176.398	176.403	176.398	176.398
Crown @ ¢ Construction El. "Tc"	176.516	176.521	176.516	176.516

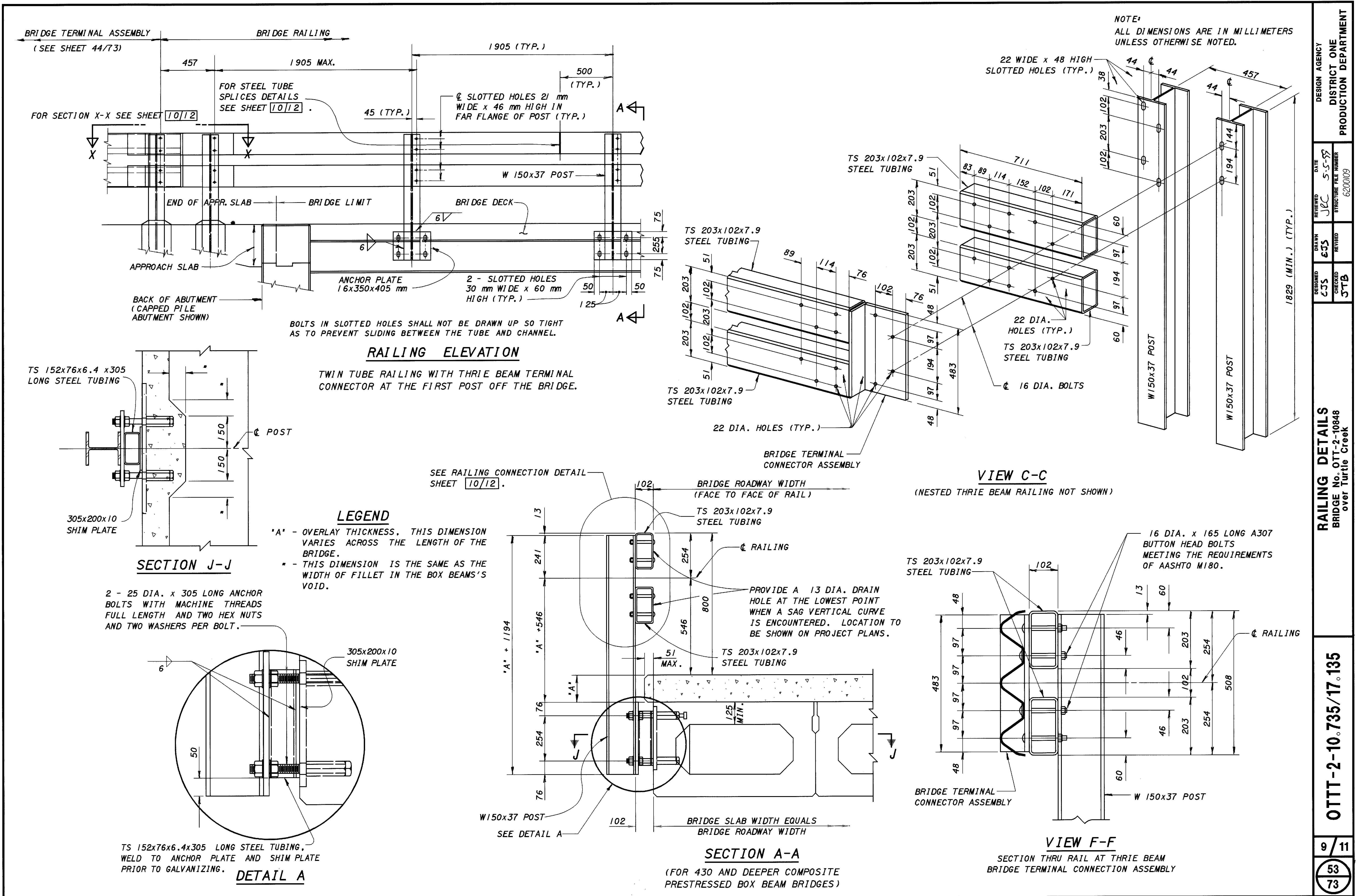
(*) Screeed Elevations shown are for the deck surface prior to concrete placement. Allowance has been made for anticipated calculated dead load deflections.



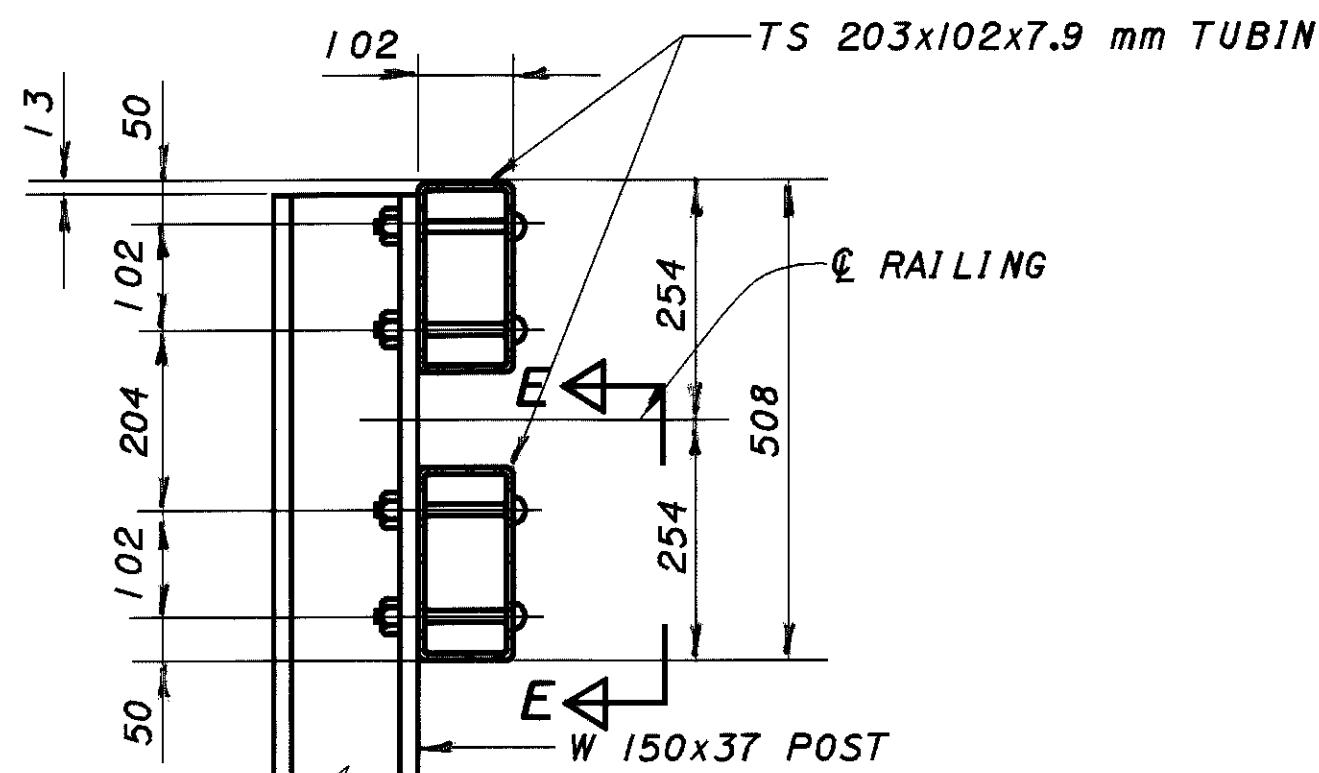
SECTION X-X

INTEGRAL BACKWALL DETAIL

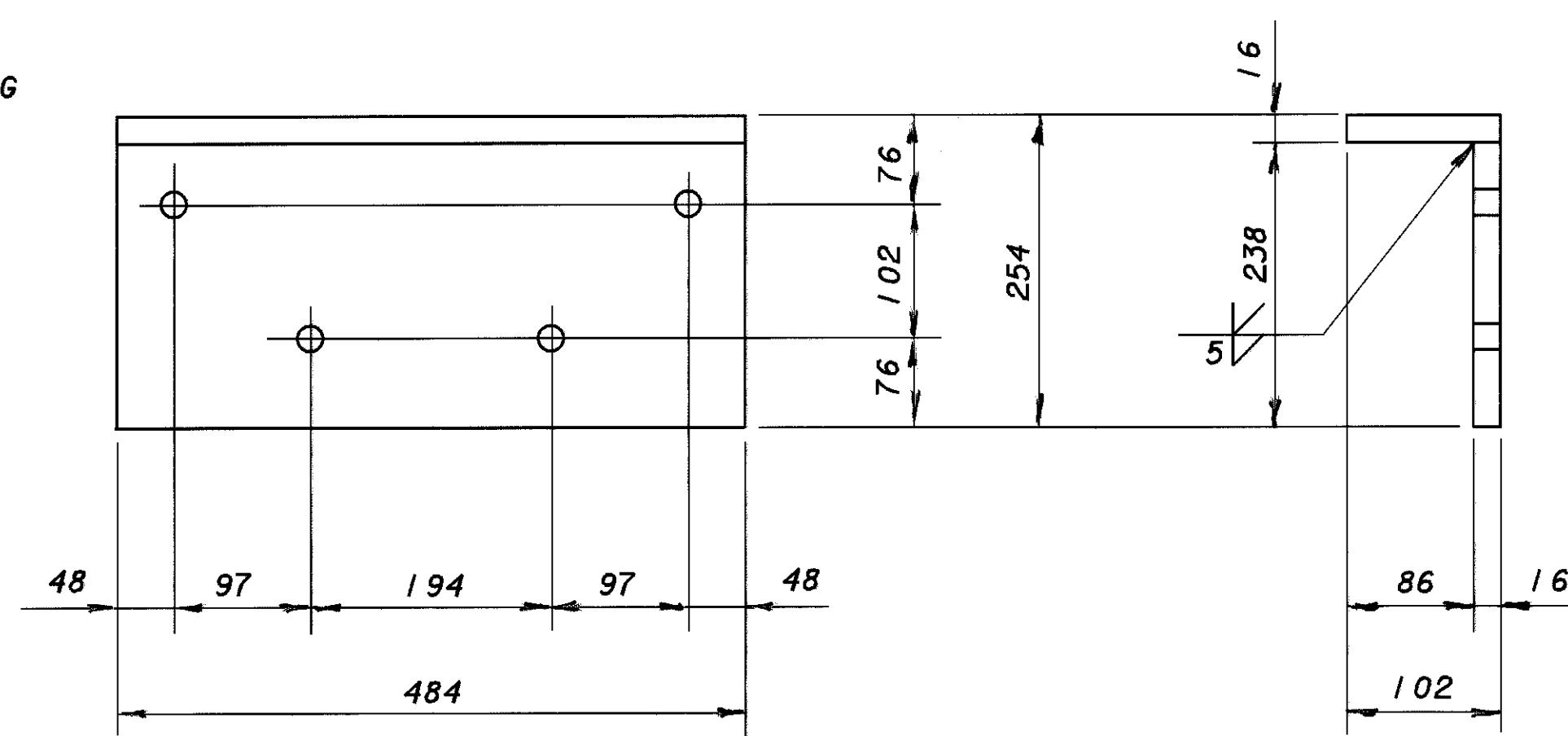
For Further Detail Information See Sheet **4/11**



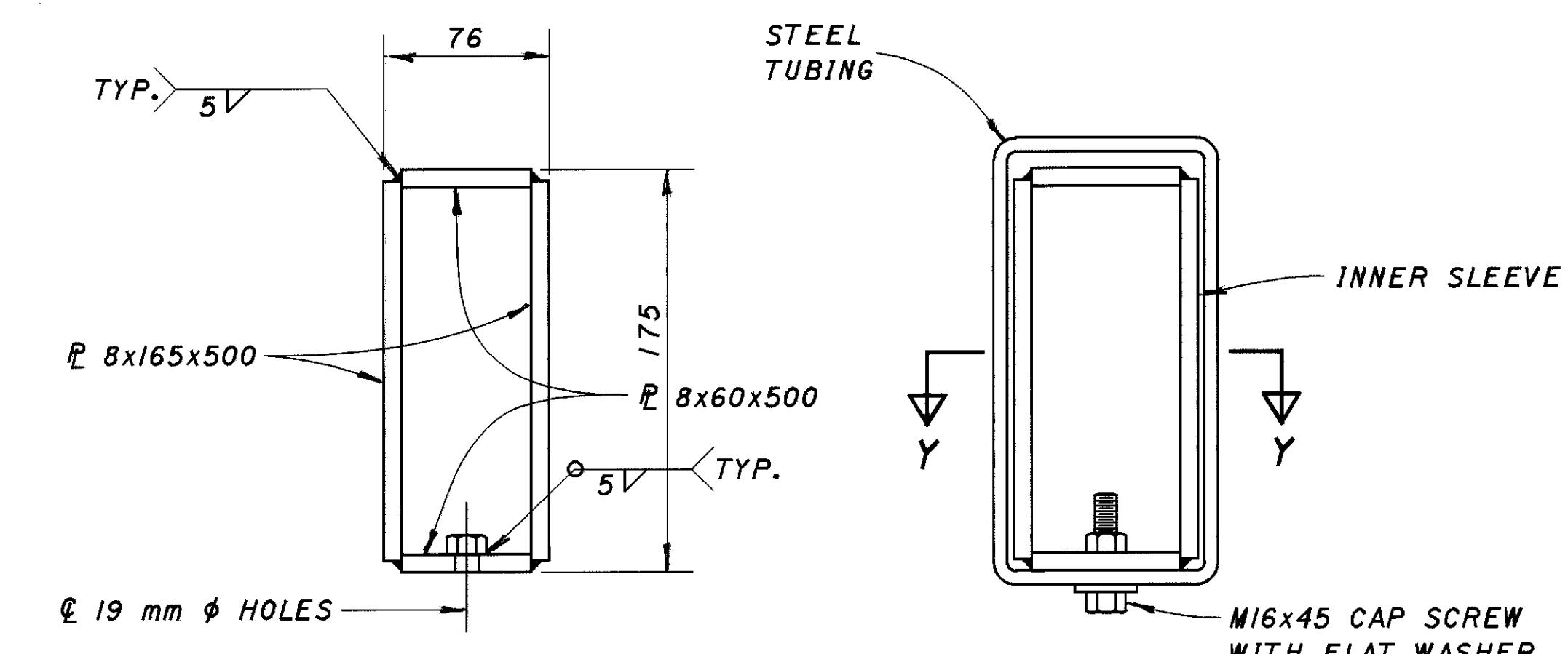
NOTE:
ALL DIMENSIONS ARE IN MILLIMETERS
UNLESS OTHERWISE NOTED.



TYPICAL SECTION THRU RAIL USING THRIE BEAM TERMINAL CONNECTOR

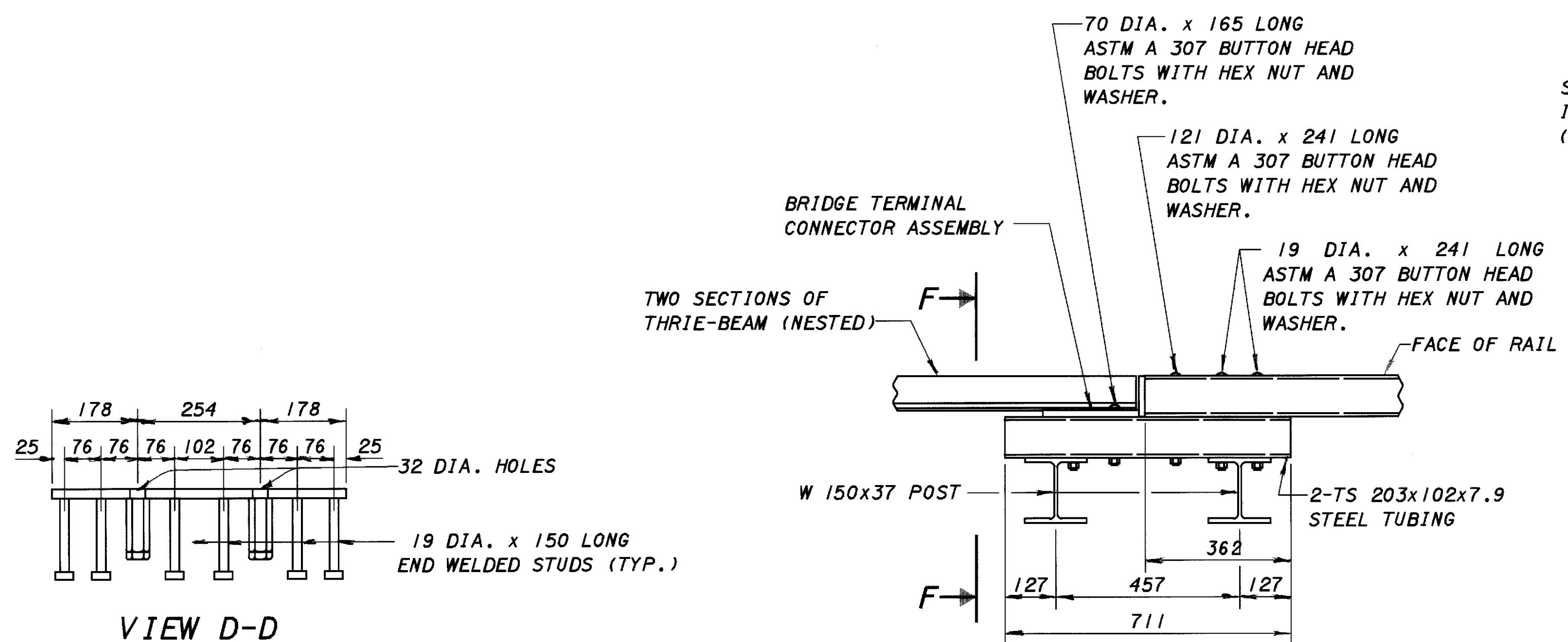


BRIDGE TERMINAL CONNECTOR ASSEMBLY

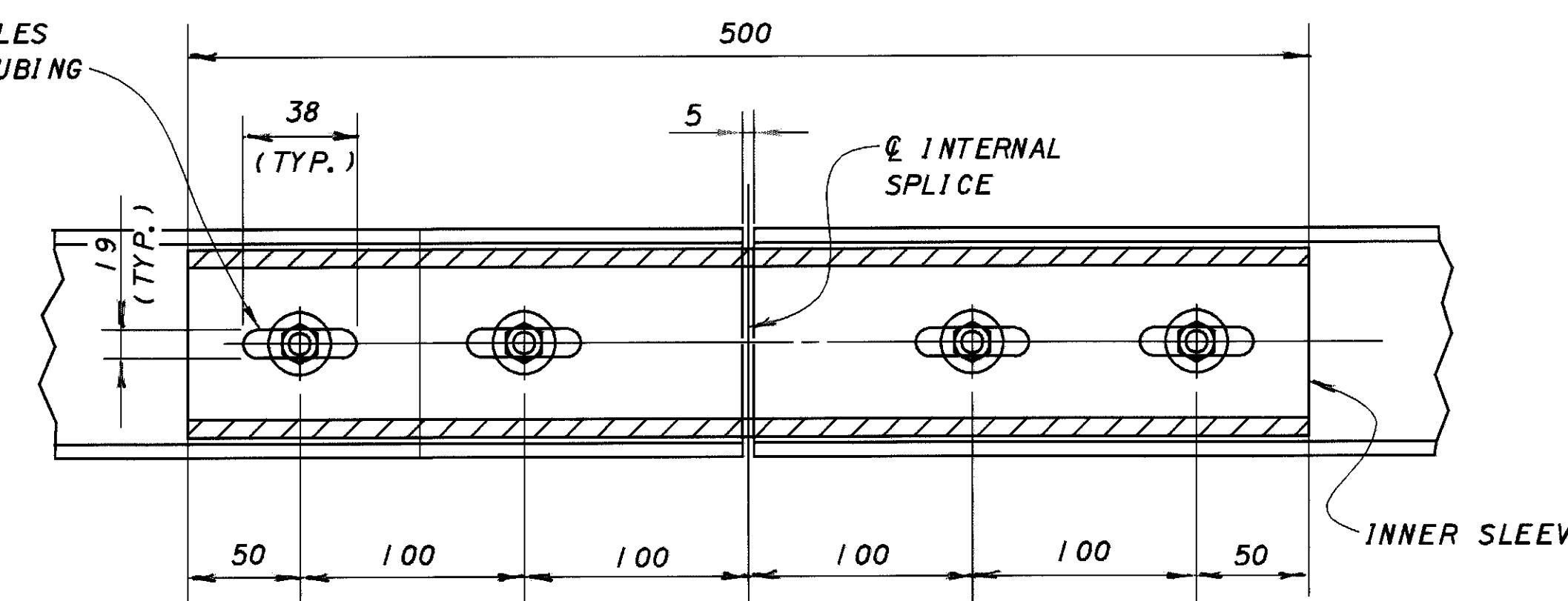


FINISHED DIMENSIONS

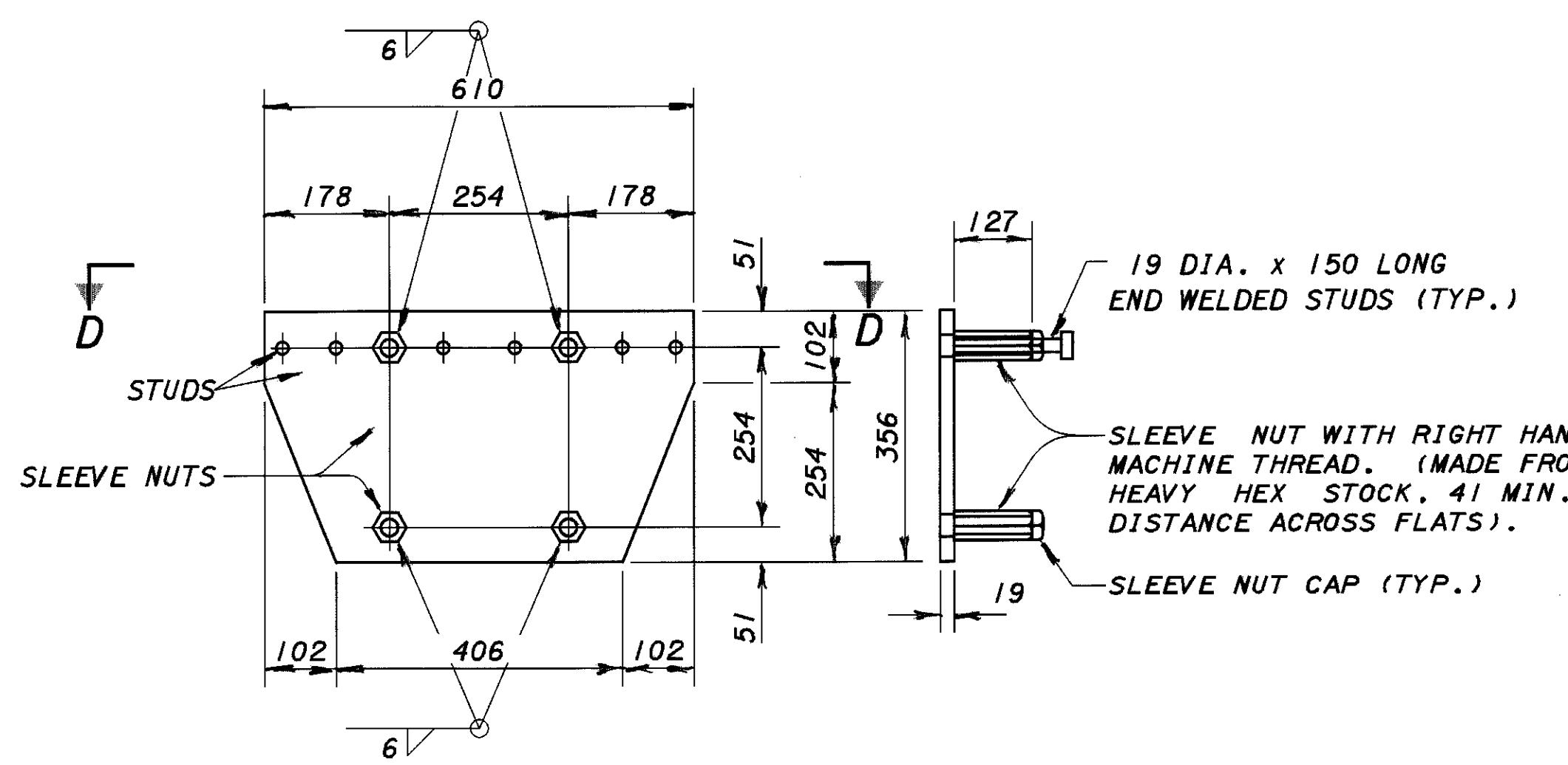
SECTION THRU SPLICE



VIEW D-D

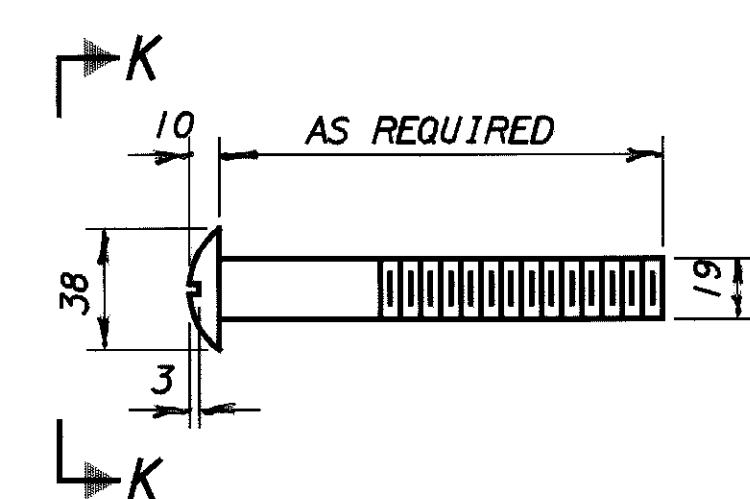


SECTION Y-Y

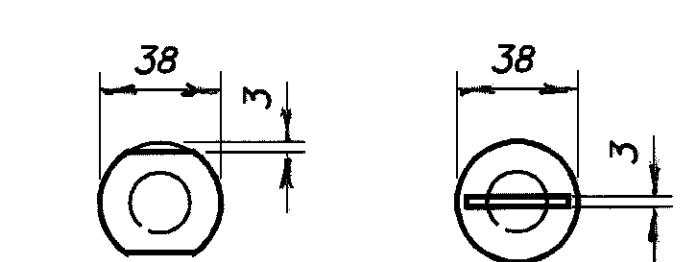


ELEVATION

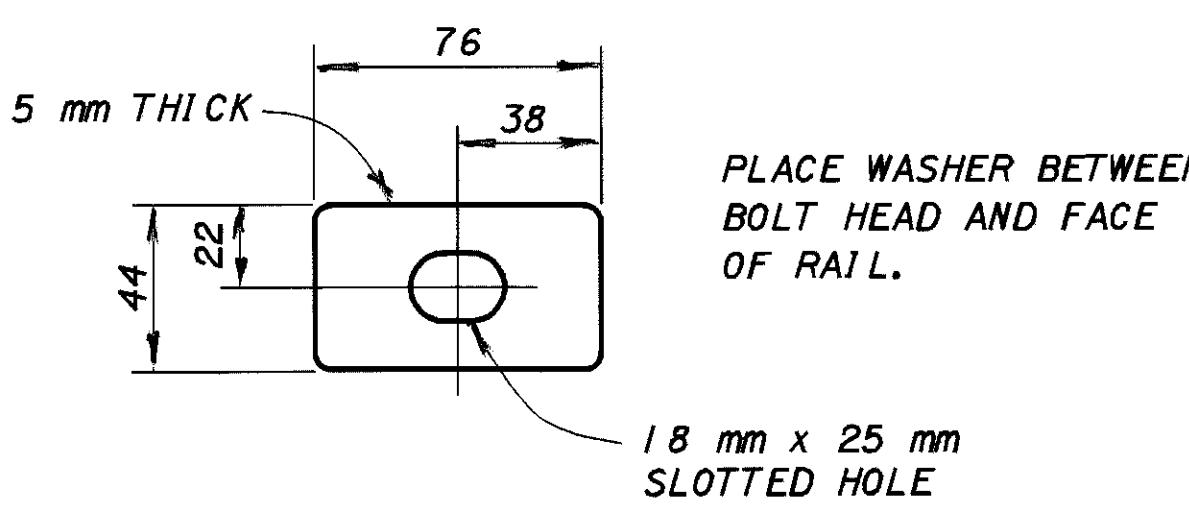
END VIEW



DETAIL OF 19 DIA. ROUND HEAD BOLT



WITHOUT SLOT **WITH SLOT**
OR RECESS



SPECIAL WASHER

DESIGNED	DRAWN	REVIEWED	DATE	DESIGN AGENCY
EJ5	EJ5	JCC	5-5-95	DISTRICT ONE
CHECKED	REVISED			PRODUCTION DEPARTMENT
				6200009

RAILING DETAIL
BRIDGE No. OTT-2-10848
over Turtle Creek

077-3-10735 / 17 135

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