

UNDERGROUND UTILITIES

Contact Two Working Days

Before You Dig

OHIO811, 8-1-1, or 1-800-362-2764

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PLAN PREPARED BY:

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Before You Dia

STATE OF OHIO DEPARTMENT OF TRANSPORTATION

SUM-76-8.24 SUM-77-9.74 **SUM-8-0.00**

CITY OF AKRON

SUMMIT COUNTY

INDEX OF SHEETS: TITLE SHEET STRUCTURES OF 20' SUM-77-1181

1 2-17

ATTACHMENTS: FINAL GEOTECHNICAL REPORT MANUFACTURER'S TRUSS PLANS

SUPPLEMENTA SPECIFICATIO		RUCTION DRAWINGS	STANDARD CONSTI			
800-2019 10/16/				7/8/14	RM-4.3	
				7/19/19	RM-4.4	
				1/18/19	BP-5.1	
						ENGINEERS SEAL:
1						FOR STRUCTURES OVER
						20'
-						AND TE OF OH
SPECIAL						★ KELLY D CHRISMAN 68020
PROVISIONS						P 68020
-						SONAL ENGLASS
-						SIGNED: Key D. Ch.
1						DATE: 9-16-2021

DESIGN EXCEPTIONS

N/A

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DESCRIPTION

ISSUE RECORD: NO. DATE

PROJECT DESCRIPTION

Released for Construction Thomas J Powell, PE 10/06/2021

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THE AKRON CENTRAL INTERCHANGE PROJECT (PID 102329) INCLUDES RECONSTRUCTION OF IR-76 EB/WB PAVEMENT FROM SLM 11.05 (IR-76) TO SLM (IR-76) FOR APPROX. 1.16 MILES OF MAINLINE WORK. THIS PROJECT ALSO INCLUDES REPLACEMENT OF TWO FREEWAY STRUCTURES (RAMP N AND RAMP Q) ON NEW ALIGNMENTS, WIDENING OF TWO EXISTING BRIDGES (IR-76 EB OVER BROWN STREET AND IR-76 WB OVER INMAN STREET), CONSTRUCTION OF A NEW PEDESTRIAN/MULTI-USE OVERPASS SPANNING SR-8, AND CONSTRUCTION OF NOISE BARRIERS AT THE PERIMETER OF THE INTERCHANGE. PORTIONS OF SR-8, LANE O AND LANE S ARE TO BE RESURFACED. THE EXISTING IR-76 WB TO INMAN STREET AND IR-77 SB TO LOVERS LANE WILL BE REMOVED PERMANENTLY. AS WELL AS THE EXISTING LAFOLLETTE STREET BRIDGE OVER SR-8 (SUM-77-1184).

BUILDABLE UNIT 12 DESCRIPTION

THIS BUILDABLE UNIT COVERS THE REMOVAL OF THE EXISTING LAFOLLETTE ST. BRIDGE OVER IR-77/SR-8 AND CONSTRUCTION OF THE RUBBER CITY HERITAGE TRAIL PEDESTRIAN BRIDGE OVER RAMP N. LANE M. SR-8 AND LANE O.

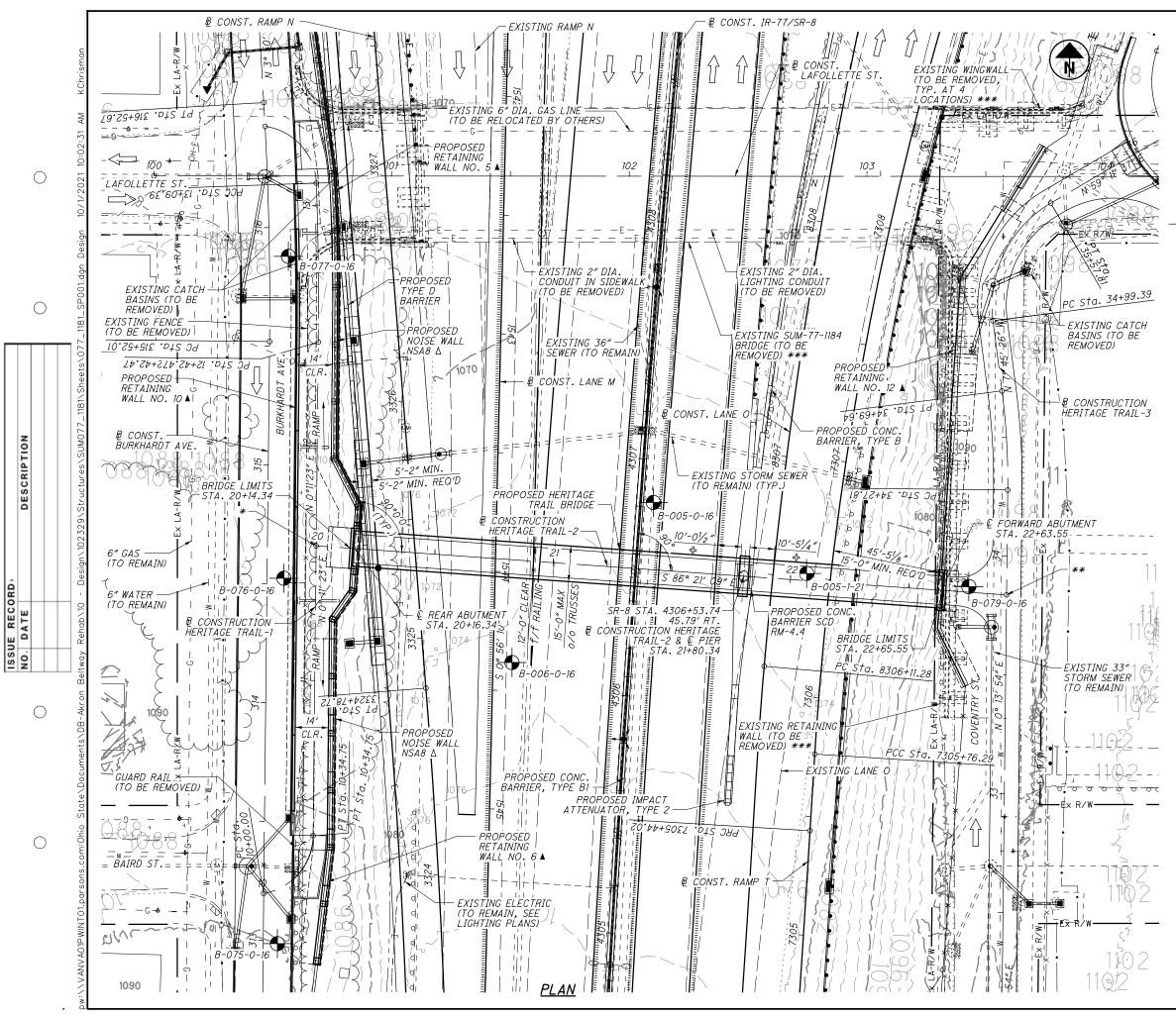
LIMITED ACCESS

THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE OHIO REVISED CODE.

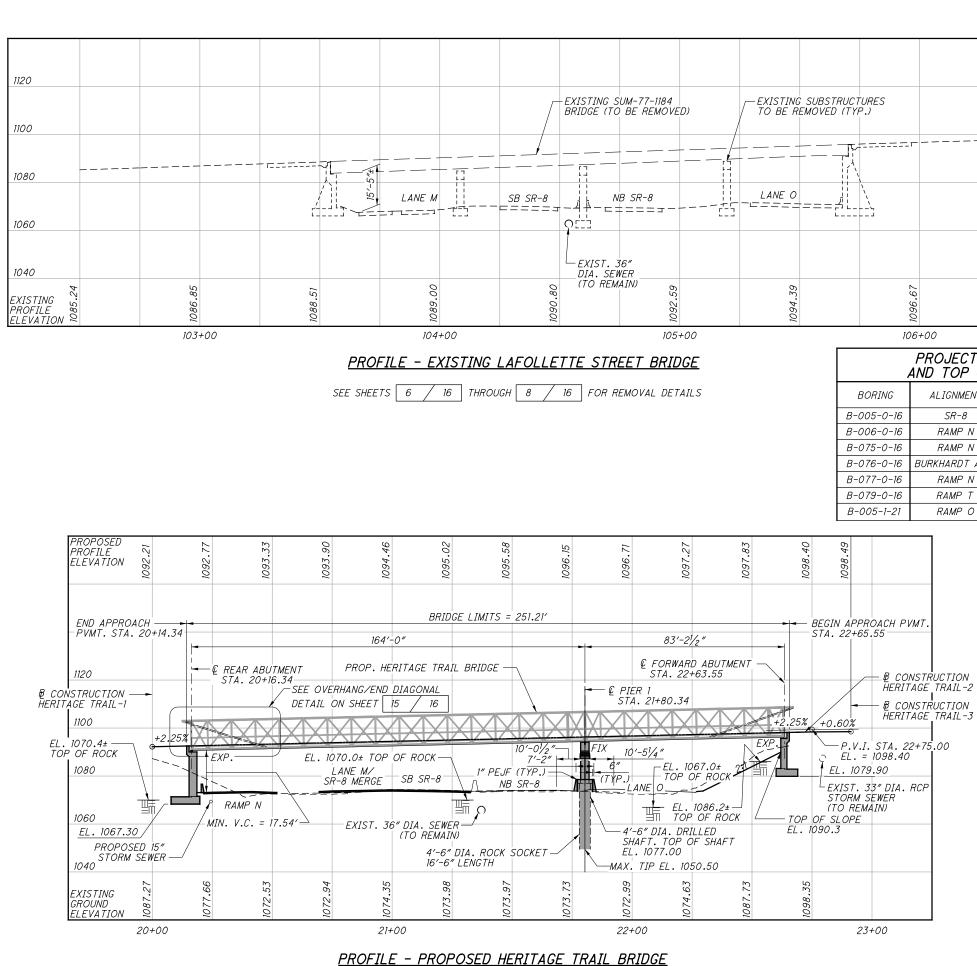
2019 SPECIFICATIONS

THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, INCLUDING SUPPLEMENTAL SPECIFICATIONS LISTED IN THE PLANS AND CHANGES LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.





Released for Construction BENCHMARK DA TAThomas J Powell, PE	
10/06/2021 BM #5 STA.105+06.46, ELEV. 1082.764, OFFSET 462.82′ RT., CMON BM #200 STA.108+40.94, ELEV. 1087.538, OFFSET 695.05′ LT., IPIN	ENCY Suite 300
NOTES EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS. SEE SHEET 2 / 16 FOR PROFILE, BORING LOCTIONS AND TOP OF ROCK ELEVATIONS.	PRIME BAR Place 1
LEGEND → PROJECT BORING LOCATION • LOCATION OF MINIMUM VERTICAL CLEARANCE • SEE SUM-76/77/8-10.99/11.54/0.00 (ATTACHMENT C) RETAINING WALL PLANS FOR DETAILS AND PAYMENT • SEE SUM-76/77/8-10.99/11.54/0.00 (ATTACHMENT C) NOISE WALL PLANS, FOR DETAILS AND PAYMENT * = € CONSTRUCTION HERITAGE TRAIL-1 STA. 11+56.70 = € CONSTRUCTION HERITAGE TRAIL-2 STA. 20+00.00 ** = € CONSTRUCTION HERITAGE TRAIL-2 STA. 22+91.19 = € CONSTRUCTION HERITAGE TRAIL-3 STA. 33+83.66	DESIGNED DRAWN REVIEWED DATE KDC KDC SAN 6/14/21 CHECKED REVISED STRUCTURE FILE NUMBER JAT 7702950
HERITAGE TRAIL-3 STA. 33+83.60 *** = SEE BRIDGE REMOVAL PLAN SHEETS 6 8 16 • = MEASURED TO THE FACE OF COLUMN EXISTING STRUCTURE TYPE: FOUR SPAN CONTINUOUS ROLLED BEAM WITH REINFORCED	SUMMIT COUNTY STA. 20+14.34 STA. 22+65.55
CONCRETE DECK ON CONCRETE PIERS AND ABUTMENTS SPANS: 53.00'±, 51.15'±, 59.85'±, 49.50'±, C/C BEARINGS ROADWAY: 44'-0"± †/† CURB, 4'-9"± SIDEWALKS (LT. & RT.) LOADING: CF-400 SKEW: VARIES (1°06'30"± TO 13°22'00"±) APPROACH SLABS: 25'-0"± (REAR AND FORWARD) ALIGNMENT: TANGENT CROWN: ¾6"/FT STRUCTURAL FILE NUMBER: 7702949 DATE BUILT: 1958 WEARING SURFACE: MONOLITHIC CONCRETE DISPOSITION: TO BE REMOVED	SITE PLAN 77-1181 RUBBER CITY HERITAGE TRAIL MP N, LANE M, S.R. 8, AND LANE O
PROPOSED STRUCTURE TYPE: PREFABRICATED STEEL TRUSS (ASTM A709 GRADE 50W, PAINTED) WITH REINFORCED CONCRETE ABUTMENTS ON SPREAD FOOTINGS AND A REINFORCED CONCRETE PIER ON A DRILLED SHAFT. SPANS: 164'-0" (© REAR ABUTMENT TO © PIER) 83'-2½" (© PIER TO © FORWARD ABUTMENT)	SITE PLA BRIDGE NO. SUM-77-1181 RUBBER BRIDGE OVER RAMP N, LANE M,
BIKEWAY: 13'-0" FACE-TO-FACE OF TRUSSES 12'-0" FACE-TO-FACE OF BRIDGE RAILINGS LOADING: AASHTO PEDESTRIAN LIVE LOAD (0.09 KSF) OR HIO-44 TRUCK SKEW: 0° ALIGNMENT: TANGENT WEARING SURFACE: CONCRETE APPROACH SLABS: NONE	SUM-76/77/8- 8.24/9.74/0.00 PID No. 101402
CROWN: NONE COORDINATES: LATITUDE: N 41°03'31.15″ LONGITUDE: W 81°30'17.41″ DECK AREA = 3224 SQ. FT. STRUCTURE FILE NUMBER: 7702950	$ \begin{array}{c} 1 \\ 1 \\ \hline 1 \\ \hline 2 \\ 17 \\ \hline \end{array} $



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DESCRIPTION

ISSUE RECORD: NO. DATE

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	ALIGNMENT	STATION	OFFSET	T.O.R. ELEVATION
	SR-8	4306+82	6′ RT.	1070.0
	RAMP N	3324+87	37′ RT.	1069.9
	RAMP N	3323+74	69′ LT.	1069.6
	BURKHARDT AVE.	314+52	7′ RT.	1070.4
	RAMP N	3326+64	43′ LT.	1069.3
	RAMP T	7306+54	58′ RT.	1086.2
	RAMP O	8306+52	14′ RT.	1067.0

COUNTY)+14.34 2+65.55 SUMMIT C STA. 20+ STA. 22+

PROFILES ier city heritage m, s.r. 8, and lan -77-1181 | AMP N, L SITE NO. SU BRIDGE

TRAIL

SUM-76/77/8-8.24/9.74/0.00 PID No.101402 2 3

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Released for Constructior Thomas J Powell, PE 10/06/2021

GENERAL NOTES:

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWINGS:

NONE

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS:

NONE

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DESCRIPTIO

RECORD

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<u>DESIGN SPECIFICATIONS:</u> THIS STRUCTURE CONFORMS TO TH BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN THIS STRUCTURE CONFORMS TO THE "LRFD ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO), 8TH EDITION, 2017, THE AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES, 2009 WITH CURRENT INTERIMS AND THE ODOT BRIDGE DESIGN MÁNUAL, 2007 EDITION WITH REVISIONS THROUGH JULY 2018, EXCEPT AS NOTED ELSEWHERE IN THE PLANS.

UNDANCY: THE PIER COLUMN WAS CONSIDERED NON-REDUNDANT DESIGN AND INCLUDES A LOAD MODIFIER EQUAL TO 1.05 IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 1.3.4.

<u>REDUNDANCY:</u> THE DRILLED SHAFT SUPPORTING THE PIER COLUMN WAS CONSIDERED NON-REDUNDANT FOR DESIGN AND INCLUDES A MODIFIED RESISTANCE FACTOR FOR TIP RESISTANCE EQUAL TO 0.40 IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 10.5.5.2.4.

<u>OPERATIONAL IMPORTANCE:</u> A LOAD MODIFIER OF 1.0 HAS BEEN ASSUMED FOR THE DESIGN OF THIS STRUCTURE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, ARTICLE 1.3.5 AND THE ODOT BRIDGE DESIGN MANUAL. 2007.

<u>DESIGN LOADING:</u> THE BRIDGE DESIGN SHALL BE BASED ON A COMBINATION OF THE FOLLOWING LOADS WHICH WILL PRODUCE MAXIMUM CRITICAL MEMBER STRESSES:

ONE AASHTO HIO-44 TRUCK. CONSIDERATION OF DYNAMIC LOADING IS NOT REQUIRED.

A PEDESTRIAN LIVE LOAD OF 90 PSF NOT TO BE USED IN CONJUNCTION WITH THE H10-44 TRUCK LOADING.

64.3 PSF WIND LOAD ON THE FULL HEIGHT OF THE BRIDGE, AS IF ENCLOSED (SEE NOTE BELOW).

TRAFFIC SIGNALS).

DESIGN DATA:

CONCRETE CLASS QC2 - COMPRESSIVE STRENGTH 4.5 KSI (SUPERSTRUCTURE) (DECK) - COMPRESSIVE STRENGTH 4.0 KSI CONCRETE CLASS OCI (SUBSTRUCTURE) (ABUTMENTS AND PIER) CONCRETE CLASS QC5 - COMPRESSIVE STRENGTH 4.5 KSI (DRILLED SHAFT AT PIER)

EPOXY COATED REINFORCING STEEL - MINIMUM YIELD STRENGTH 60 KSI - SHAPES AND PLATES - ASTM A709 GRADE 50W - YIELD STRENGTH 50 KSI STRUCTURAL STEEL TUBES ASTM A847 GRADE 50W - YIELD

STRENGTH 50 KSI

DECK PROTECTION METHOD:

FPOXY COATED REINFORCING STEEL 2" CONCRETE COVER CLASS QC2 CONCRETE

<u>MAINTENANCE OF TRAFFIC:</u> MAINTENANCE OF TRAFFIC FOR THE STRUCTURE WORK SHALL BE COORDINATED WITH THE OVERALL PROJECT. REFER TO MAINTENANCE OF TRAFFIC NOTES AND DETAILS ELSEWHERE IN THE PLANS.

FOUNDATION BEARING RESISTANCE: ABUTMENT FOOTINGS, AS DESIGNED, PRODUCE A MAXIMUM SERVICE LOAD PRESSURE OF 11.46 KIPS PER SQUARE FOOT AND A MAXIMUM STRENGTH LOAD PRESSURE OF 16.02 KIPS PER SQUARE FOOT AT THE REAR ABUTMENT AND PRODUCE A MAXIMUM SERVICE LOAD PRESSURE OF 6.99 KIPS PER SQUARE FOOT AND A MAXIMUM STRENGTH LOAD PRESSURE OF 9.24 KIPS PER SQUARE FOOT AT THE FORWARD ABUTMENT. THE FACTORED BEARING RESISTANCE IS 18.0 KIPS PER SQUARE FOOT AT THE REAR ABUTMENT AND 36.0 KIPS PER SQUARE FOOT AT THE FORWARD ABUTMENT.

<u>FOOTINGS AT THE ABUTMENTS</u> SHALL EXTEND A MINIMUM OF 3" INTO BEDROCK OR TO THE ELEVATION SHOWN, WHICHEVER IS LOWER.

DRILLED SHAFTS:

THE FOLLOWING TABLE SUMMARIZES THE DRILLED SHAFT FACTORED LOADS AND FACTORED RESISTANCES PROVIDED AT EACH SUBSTRUCTURE. THE MAXIMUM FACTORED LOAD IS FULLY SUPPORTED BY THE DRILLED SHAFT IN TIP RESISTANCE, IGNORING ANY

CONTRIBUTION FROM SIDE RESISTANCE. ALL INFORMATION IN THE TABLE IS GIVEN PER EACH DRILLED SHAFT.

LOCATION	MAXIMUM FACTORED LOAD (KIPS)	FACTORED TIP RESISTANCE (KIPS)	TOTAL FACTORED RESISTANCE (KIPS)	
PIER 1	797	916	916	

DRILLED SHAFT ROCK SOCKET LENGTHS AND TIP ELEVATIONS:

AT PIER 1, IN THE EVENT THAT THE TOP OF ROCK ELEVATION ENCOUNTÉRED IN THE FIELD IS LOWER THAN SPECIFIED ON THE PLANS, THE TIP ELEVATION SHOWN ON THE PLANS SHALL BE LOWERED BY AN AMOUNT EQUAL TO THE DIFFERENCE BETWEEN THE SPECIFIED ELEVATION IN THE PLANS AND THE TOP OF ROCK ELEVATION ENCOUNTERED IN THE FIELD.

ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE):

SEE AESTHETIC PLANS FOR SEALING REQUIREMENTS. SEAL THE RETAINING WALLS AND REAR ABUTMENT WITH AN EPOXY-URETHANE SEALER MATCHING FEDERAL COLOR STANDARD 27769, GENERAL/ LIGHT NEUTRAL. SEAL THE FORWARD ABUTMENT WITH AN EPOXY-URETHANE SEALER MATCHING FEDERAL COLOR STANDARD 23522. GENERAL/TAN. IN ADDITION TO THE LIMITS OF SEALING SHOWN ON THE PLANS, SEAL ALL EXPOSED CONCRETE SURFACES OF THE PROPOSED PIÉR, INCLUDING THE TOP HORIZONTAL SURFACES OF THE PIER CAP. REFER TO CMS 516.07 FOR SEALING REQUIREMENTS AT BEARING AREAS.

ITEM 516 - PREFORMED ELASTOMERIC COMPRESSION JOINT SEAL, AS PER PLAN:

THIS WORK SHALL CONSIST OF FURNISHING AND INSTALLING PREFORMED ELASTOMERIC COMPRESSION JOINT SEALS AT THE SUPERSTRUCTURE DECK SLAB EXPANSION JOINTS LOCATED AT THE NOTE: Pz = (0.00256)(0.90)(0.85)(1.14)(120)² (2.0) = 64.3 PSF (PER 3.4 OF AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES AND 3.8.1 OF AASHTO LRFD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS. I UMINATEES AND TRAFFIC SIGNAL SUP REAR ABUTMENT AND FORWARD ABUTMENT. THE MULTI-CELLULAR,

FOLLOWING OR AN ENGINEER APPROVED EQUAL:

THE D.S. BROWN COMPANY (TYPE JP SERIES SEALING SYSTEM) 300 EAST CHERRY STREET NORTH BALTIMORE. OHIO 45872 PHONE: 419-257-3561 www.dsbrown.com

WATSON BOWMAN ACME CORPORATION (JEENE BRIDGE SERIES TYPE FW PROFILE) 95 PINEVIEW DRIVE AMHERST, NEW YORK 14228 PHONE: 716-691-7566 www.wbacorp.com

ERIE METAL SPECIALTIES (TYPE JP SERIES SEALING SYSTEM) 13311 MAIN ROAD AKRON, NEW YORK 14001 PHONE: 716-542-3991 www.eriemetal.com

THE TOP SURFACE OF THE SEAL SHALL BE NON-SLIP AND COMPL WITH ADA GUIDELINES WHEN INSTALLED. FIELD OR SHOP FABRICATE THE JOINT SEAL, AS REQUIRED BY THE EXPANSION JOINT MANUFACTURER, TO CONFORM TO THE DIRECTIONAL CHANGES UPTURNS ARE REQUIRED AT THE CURBS TO PROVIDE A WATERTIGHT GUTTER SEAL .

THE NEOPRENE SEALS SHALL BE BONDED TO THE CONCRETE SURFACES WITH AN EPOXY BASED STRUCTURAL ADHESIVE ACCORDING THE MANUFACTURER'S REQUIREMENTS.

PREPARE ALL SURFACES AND INSTALL THE SEAL ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS AND UNDER THE DIRECTION OF THE MANUFACTURER'S RECOMMENDATIONS.

THE DEPARTMENT WILL MEASURE THE EXPANSION JOINT FOR PAYMENT PURPOSES BY THE NUMBER OF FEET HORIZONTALLY ALONG THE JOINT CENTERLINE FROM EDGE OF DECK TO EDGE OF DECK.

<u>UNIT</u>	<u>DESCRIPTION</u>
FOOT	PREFORMED ELASTOMERIC COMPRESSION JOINT SEAL, AS PER PLAN

<u>ITEM 524 - DRILLED SHAFTS, 54″ DIAMETER, ABOVE BEDROCK, AS PER PLAN:</u>

THE AGGREGATE SHALL BE 3/6" NOMINAL MAXIMUM SIZE.

524 - DRILLED SHAFTS, 54" DIAMETER, INTO BEDROCK, AS PER PLAN:

ALL MEMBERS OF THE MAIN TRUSSES AND BRACING SYSTEM SHALL BE FABRICATED FROM SQUARE AND/OR RECTANGULAR STRUCTURAL STEEL THE AGGREGATE SHALL BE 3/8" NOMINAL MAXIMUM SIZE. TUBING. ALTERNATIVELY, FLOOR BEAMS MAY BE WIDE FLANGE OR CHANNEL SHAPES AS REQUIRED BY DESIGN OR SPECIFIC DETAIL. ALL STRUCTURAL STEEL MEMBERS SHALL HAVE A MINIMUM THICKNESS OF MATERIAL OF AT LEAST ¼- INCH. FIELD SPLICES AND CONNECTIONS SHALL BE BOLTED TYPE WITH HIGH STRENGTH ASTM A325 TYPE I ITEM 524 – DRILLED SHAFTS, MISC.: THERMAL INTEGRITY PROFILER (T.I.P.) WIRE CABLE TESTING OF DRILLED SHAFTS: BOLTS, HEAVY HEX NUTS AND WASHERS. SEE NOTE TITLED "DESIGN PERFORM INTEGRITY TESTING ON THE DRILLED SHAFT SUPPORTING THE SINGLE-COLUMN PIER BY THERMAL INTEGRITY PROFILING (T.I.P.). DATA" FOR ADDITIONAL STEEL MEMBER SPECIFICATIONS.

PERFORM TIP TESTING PER ASTM D7949 "STANDARD TEST METHODS FOR THERMAL INTEGRITY PROFILING OF CONCRETE DEEP FOUNDATIONS", METHOD B, AND PER THE PROJECT SPECIAL PROVISIONS.

<u>ITEM SPECIAL - S</u> <u>SUPERSTRUCTURE</u>: - STRUCTURE MISC.: PREFABRICATED PAINTED STEEL

GENERAL:

<u> ITEM</u>

516

THE MESH SHALL BE 1-INCH X 1-INCH ON CENTER SQUARE OPENINGS, 0.192-INCH THICK (6 GAUGE) WIRE DIAMETER WELDED WIRE MESH AS THIS WORK INCLUDES THE COMPLETE DESIGN. FABRICATION AND ERECTION OF A PAINTED, WELDED STEEL TRUSS PEDESTRIAN BRIDGE PROVIDED BY BANKER WIRE OR AN ENGINEER APPROVED EQUAL. THF SUPERSTRUCTURE AS NOTED HEREIN AND SHOWN ON THE PLANS. ANY MESH SHALL BE ZINC CHROMATE PLATED WITH A ZINC RICH PRIMER NOTES OR SPECIFICATIONS LISTED HEREIN OR SHOWN ON THE PLANS POWDER COAT AND A POLYESTER POWDER TOP COAT. TOP COAT SHALL BE REGARDED AS MINIMUM STANDARDS FOR DESIGN AND COLORS ARE INDICATED BELOW. THE TYPICAL MESH ENCLOSURES SHALL CONSIST OF FOUR SEPARATE PANELS IN EACH TRUSS BAY AND SHALL INFILL THE ENTIRE AREA FROM THE BOTTOM CHORD TO THE TOP CHORD, LEAVING A 1/2-INCH MAXIMUM GAP BETWEEN PANELS AND TRUSS MEMBERS. SINCLE PANELS WITH SUPPLEMENTAL MID-BAY CONSTRUCTION. THE BRIDGE SHALL BE MANUFACTURED BY ONE OF THE FOLLOWING: U.S. BRIDGE VERTICAL MEMBERS WILL NOT BE PERMITTED. THE MESH SHALL BE SANDWICHED BETWEEN TWO 1/4-INCH X 2-INCH PLATES AROUND THE PERIMETER TO FORM PANELS' AND SHALL BE BOLTED WITH PHONE: 888-USBRIDGE (872-7434) STAINLEES STEEL BOLTS, NUTS, WASHERS AND OTHER FASTENERS STAINLESS STEEL BOLTS, NUTS, WASHERS AND OTHER FASTENERS TO CLIP ANGLES SHOP WELDED BY THE PRE-ENGINEERED BRIDGE MANUFACTURER TO THE TRUSS MEMBERS. PANELS FABRICATED WITH PERIMETER 1/4" WIDE BY 1/2" DEEP BY 1/8" THICK STEEL U-CHANNELS WITH THE MESH WELDED TO THE U-CHANNELS MAY BE USED INSTEAD OF THE MECHT AND MEMORY PERIMENT. <u>www.usbridge.com</u> MAIN TRUSSES SHALL BE DESIGNED AS PARALLEL CHORD PRATT STYLE OF THE MESH SANDWICHED BETWEEN TWO PLATES. SPECIFIC DETAILS SHALL BE PROVIDED TO THE ENGINEER FOR APPROVAL. TYPF 1 VERTICAL MEMBERS. TYPE 2 PANELS SHALL BE POWDER COAT PAINTED GRAY SAE AMS-STD-595 #16515 PRIOR TO ATTACHING TO THE TRUSS.

201 WHEELING AVENUE P.O. BOX 757 CAMBRIDGE, OHIO 43725

DESIGN:

TRUSSES WITH VERTICAL END POSTS, TWO DIAGONALS PER BAY, UPPER LATERAL BRACING AND STRUTS AS SHOWN ON THE PLANS. OVERALL SECTION GEOMETRY SHALL FOLLOW THE DIMENSIONS SHOWN ON THE TYPICAL SECTION ON SHEET OF . SUPERSTRUCTURE PANELS SHALL HAVE THE MESH GRID ORIENTED PARALLEL AND PERPENDICULAR TO THE TRUSS VERTICAL MEMBERS. TYPE 1 PANELS SHALL BE POWDER COAT PAINTED RED SAE AMS-STD-595 #11350 PRIOR TO ATTACHING TO THE TRUSS. TYPE 2 PANELS SHALL HAVE THE MESH GRID ORIENTED AT 45 DEGREE ANGLES TO THE TRUSS DESIGN SHALL CONFORM TO THE AASHTO LRFD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES WITH CURRENT INTERIMS. STRUCTURAL DESIGN SHALL BE PERFORMED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OHIO AND DONE WITH RECOGNIZED ENGINEERING PRACTICES AND PRINCIPLES. DESIGN LIVE LOAD SHALL BE AS STIPULATED IN THE NOTE TITLED "DESIGN LOADING". THE PROJECTED AREA FOR WIND FORCES ACTING ON THE ADDITIONAL MESH PANELS, LOCATED AT EACH TRUSS ABUTMENT END POST, PERPENDICULAR TO THE CENTERLINE OF THE TRUSS SHALL BE MESH PANELS SHALL NOT BE LESS THAN 20% OF THE GROSS AREA OF THE MESH PANEL. MAIN LOAD CARRYING TENSION MEMBERS SHALL INCLUDED WITH THE TRUSS TO FILL THE GAP BETWEEN THE TRUSS MEET THE CHARPY-V NOTCH TOUGHNESS REQUIREMENTS OF C&MS END POSTS AND THE ADJACENT RETAINING WALL/NOISE WALL END THE WELDED CONNECTIONS BETWEEN THE BOTTOM CHORDS 711.01. ΔND POSTS AT THE REAR ABUTMENT AND THE RETAINING WALL/CHAIN THE FLOOR BEAMS SHALL BE DESIGNED AN THE BETTING WITH SAN THE FLOOR BEAMS SHALL BE DESIGNED AS TAXED CONNECTIONS (NOT PINNED). THE BRIDGE SPANS SHALL BE DESIGNED WITH A VERTICAL CAMBER AT MIDSPAN EQUAL TO 100% OF THE FULL DEAD LOAD DEFLECTION, INCLUDING THE WEIGHT OF THE DECK, SO THAT THE LINK FENCE END POSTS AT THE FORWARD ABUTMENT. MESH PANELS LINK FENCE END FOSTS AT THE FORWARD ABOTMENT. MICHT AINLY WITH MESH SANDWICHED BETWEEN TWO PLATES OR A PLATE AND AN ANGLE AND ATTACHED TO THE TRUSS END POSTS WITH BOLTS AND CLIP ANGLES SHALL BE FABRICATED AS INDICATED IN THE PREVIOUS PARAGRAPH. THE PANEL'S GRID ORIENTATION AND COLOR SHALL END THE SAME AS TYPE 2 PANELS. PANELS SHALL NOT BE ATTACHED BRIDGE CONFORMS TO THE PROPOSED PROFILE GRADE LINE IN ITS FINAL DEFLECTED POSITION. EACH SPAN SHALL BE DESIGNED TO ACCOMMODATE A TEMPERATURE DIFFERENTIAL OF 150 DEGREES THE NOISE WALL END POSTS OR CHAIN LINK FENCE END POSTS FAHRENHEIT (FROM -30 DEGREES TO 120 DEGREES). THE NEUTRAL PROVIDE A $\frac{1}{2}$ -INCH GAP MINIMUM BETWEEN THE MESH PANELS AND TEMPERATURE SHALL BE 60 DEGREES FAHRENHEIT. THE NOISE WALL END POSTS OR CHAIN LINK FENCE END POSTS. VERTICALLY THE MESH PANELS SHALL FILL THE GAP FROM THE TOP OF THE DECK TO THE TOP OF THE TRUSS. ALL PANEL DIMENSIONS SUBSTRUCTURE DESIGN AND ELEVATIONS ARE BASED ON ASSUMED SHALL BE FIELD VERIFIED PRIOR TO FABRICATION TO ENSURE THE PANEL FITS THE ACTUAL GAP DIMENSIONS. SEE SHEET 14 OF 16 F THE CENTERLINE OF TRAIL TO THE CONCRETE BEARING SEATS, THE ADDITIONAL INFORMATION.

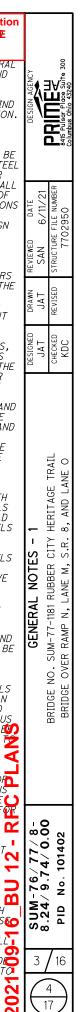
VERTICAL DIMENSIONS (DECK THICKNESS, FLOOR BEAM/BOTTOM CHORD THICKNESS AND BEARING HEIGHT) FROM THE FINISHED DECK AT CONTRACTOR SHALL ADJUST THE SUBSTRUCTURE BEARING SEAT ELEVATIONS SHOWN ON THE PLANS BASED ON THE ACTUAL VERTICAL THE TRUSS TOP CHORDS SHALL BE EXTENDED AND BRACED TO PROVIDE DECORATIVE OVERHANGS AT THE ABUTMENTS. SEE SHEET 15 OF 16 FOR DETAILS. DECORATIVE NON-LOAD CARRYING END DIMENSIONS FROM THE FINISHED DECK AT THE CENTERLINE OF THE TRAIL TO THE BOTTOM OF THE END FLOORBEAM PLUS BEARING THICKNESS. SEE SUBSTRUCTURE SHEETS FOR REINFORCING STEEL ADJUSTMENT NOTES. THE BRIDGE MANUFACTURER SHALL DETERMINE DIAGONALS SHALL BE PROVIDED ON EACH SIDE OF THE TRUSS AT THE ABUTMENTS. SEE SHEET 15 OF 16 FOR DETAILS. THE LOCATION, QUANTITY, DIAMETER, GRADE, FINISH (CORROSION

Released for Construction RESISTANT HOT DIP GALVANIZED STEEL OR STATIONES SPECIAL ME EMBEDMENT OF BEARING ANCHOR BOLTS DESIGNED TO RESIST ALL HORIZONTAL AND UPLIFT FORCES TO BE TRANSFERRED FROM THE SUPERSTRUCTURE TO THE SUBSTRUCTURE. IN ADDITION TO LATERAL WIND LOADS. THE CONNECTION BETWEEN THE SUPERSTRUCTURE AND THE SUBSTRUCTURE SHALL BE DESIGNED TO RESIST A LATERAL SEISMIC LOAD IN THE RESTRAINED DIRECTION (NORMAL TO THE BRIDGE) EQUAL TO 20% OF THE SUPERSTRUCTURE DEAD LOAD. WIND LOADS AND SEISMIC LOADS SHALL NOT BE APPLIED IN CONJUNCTION

MATERIALS:

VERTICAL MESH PANELS MUST BE DESIGNED TO FIT WITHIN THE TRUSS HORIZONTAL, VERTICAL AND DIAGONAL MEMBERS AND BE LOCATED BEHIND THE RUB RAILS AND WITHIN THE VARIOUS MEMBERS ON EACH SIDE OF THE TRUSS SO AS TO NOT PROTRUDE BEYOND THE VERTICAL FACES OF THE TRUSS MEMBERS. HORIZONTAL MESH PANELS OR FENCING AT THE TOP OF THE TRUSS CAN BE LOCATED ON THE TOP OF THE UPPER CHORDS, LATERAL BRACING AND STRUI MEMBERS.

FENCE FABRIC (LOCATED AT THE TOP OF THE TRUSS ONLY, NOT n SIDES) SHALL CONSIST OF 1-INCH DIAMOND MESH USING 0.120-INCH THICK (11 GAUGE) WIRE DIAMETER CONFORMING TO ASTM F668 CLAS 2A. THE PVC COATING SHALL BE RED SAE AMS-STD-595 #11350. SELVAGES SHALL BE KNUCKLED AT BOTH ENDS. FABRIC TIES SHALL BE PVC COATED THE SAME AS THE FENCE FABRIC. FENCE COMPONENT ATTACHMENTS TO THE TRUSS MEMBERS SHALL BE MADI WITH CONNECTIONS THAT DO NOT ALLOW WATER INFILTRATION INTO THE STRUCTURAL TUBES.



GENERAL NOTES (CONTINUED):

STEEL RUB RAILS SHALL BE HOLLOW STRUCTURAL SECTIONS (HSS) PER ASTM 1085 AND AS INDICATED ON THE PLANS. RUB RAILS SHALL BE PAINTED WITH THE SAME SYSTEM USED TO PAINT THE TRUSS. RUB RAILS SHALL BE WELDED TO THE TRUSS VERTICALS AS INDICATED ON THE PLANS.

FABRICATION:

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THE FABRICATOR SHALL MEET LEVEL 6 QUALIFICATIONS PER C&MS 513.03.

WELDERS SHALL BE PROPERLY ACCREDITED EXPERIENCED OPERATORS, EACH OF WHOM SHALL SUBMIT CERTIFICATION OF SATISFACTORILY PASSING AWS STANDARD QUALIFICATION TESTS FOR ALL POSITIONS WITH UNLIMITED BASE METAL THICKNESS AND HAVE AT LEAST 6 MONTHS EXPERIENCE IN WELDING TUBULAR AND OTHER STRUCTURES AND WHO HAVE DEMONSTRATED THE ABILITY TO MAKE UNIFORM GOOD WELDS MEETING THE SIZE AND TYPE OF WELD REQUIRED.

SPECIAL ATTENTION SHALL BE GIVEN TO DEVELOPING SUFFICIENT WELD THROATS ON TUBULAR MEMBERS. WELD DETAILS SHALL BE IN ACCORDANCE WITH AWS DI.I. FILLET WELDS WHICH RUN ONTO THE RADIUS OF A TUBE SHALL BE BUILT-UP TO OBTAIN THE FULL THROAT THICKNESS.

THE BRIDGE SHALL BE INSPECTED BY A CERTIFIED WELD INSPECTOR (CWI) THAT IS QUALIFIED UNDER THE AWS QC-1 PROGRAM. THIS INSPECTION SHALL INCLUDE AS A MINIMUM REQUIREMENT THE FOLLOWING: REVIEW OF SHOP DRAWINGS, WELD PROCEDURES, WELDER QUALIFICATIONS AND WELD TEST REPORTS, VISUAL INSPECTION OF WELDS AND VERIFICATION OF OVERALL DIMENSIONS AND GEOMETRY OF BRIDGE. A REPORT SHALL BE PRODUCED INDICATING THE ABOVE ITEMS WERE REVIEWED. THE REPORT SHALL BE SIGNED BY THE CWI, SIGNIFYING COMPLIANCE WITH AWS D1.1 CODES.

WEEP HOLES:

THE FOLLOWING PROCEDURES ARE FOR INSTALLATION OF WEEP HOLES IN SPLICED TRUSSES OR OTHER STRUCTURAL MEMBERS TO PROVIDE POSITIVE DRAINAGE FOR ANY MEMBER THAT COULD HOLD WATER EITHER DURING CONSTRUCTION OR DURING SERVICE. WEEP HOLES SHALL BE PROVIDED AT THE LOWEST POINT OF THE MEMBER.

WHEN A WEEP HOLE IS REQUIRED IN THE TOP CHORD (AT AN END PORTAL OR IF THE END VERTICAL IS EXTENDED UP WITH CAP AND/OR STIFFENER PLATES), A ⁷/₁₆-INCH DIAMETER WEEP HOLE SHALL BE PLACED AS CLOSE TO THE WELD AS POSSIBLE. EITHER BURNING THROUGH OR DRILLING IS ACCEPTABLE. REMOVE BURRS WITH A GRINDER AS REQUIRED.

IF THERE IS NOT SUFFICIENT CLEARANCE ON THE BOTTOM TUBE FACE NEAR THE WELD FOR THE HOLE, IT MAY BE PUT IN FRONT OF THE END DIAGONAL.

WHERE THE FLOOR BEAMS DO NOT HAVE OPEN ENDS AND ANY HOLE OR SCREW IS PUT IN THE FLOOR BEAM, A $\frac{7}{16}$ -INCH DIAMETER WEEP HOLE WILL BE DRILLED IN EACH END OF THE FLOOR BEAM.

THESE HOLES SHALL BE DRILLED PRIOR TO INSTALLING THE FLOOR BEAMS.

FOR THE BOTTOM CHORD AND END VERTICALS WHERE WEEP HOLES ARE REQUIRED, TORCH OR GRIND A $\frac{1}{2}$ -INCH HALF CIRCLE AT THE END OF THE MEMBER. THIS HOLE SHOULD BE TOUCHED UP WITH A DIE GRINDER SO THAT THERE ARE NO SHARP EDGES. THIS HOLE SHOULD BE AS CLOSE TO THE CENTER OF THE TUBE FACE AS POSSIBLE. DO NOT WELD AT THE HOLE.

FOR SPLICED VERTICAL DIAGONALS, DRILL A 7_{6} -INCH DIAMETER HOLE AS CLOSE TO THE BASE OF THE DIAGONAL AS POSSIBLE ON THE OUTSIDE FACE OF THE MEMBER, THEN GRIND REMAINING MATERIAL OUT TO CHORD FACE AND INSTALL WITHOUT WELDING AT THE HOLE. THIS HOLE SHOULD BE DRILLED ON A PRESS PRIOR TO INSTALLING THE DIAGONAL IN THE BRIDGE.

BEARING DEVICES:

BEARING DEVICES SHALL BE DESIGNED AND SUPPLIED BY THE BRIDGE FABRICATOR. BEARINGS SHALL MEET THE REQUIREMENTS OF C&MS 516. BRIDGE EXPANSION BEARINGS SHALL INCLUDE STAINLESS STEEL/TEFLON SLIDING SURFACES/ELASTOMERIC BEARINGS OR LAMINATED ELASTOMERIC BEARINGS AND BE DESIGNED TO ACCOMMODATE THE FULL MOVEMENT REQUIREMENTS. STAINLESS STEEL, TEFLON AND ELASTOMERIC SURFACES SHALL NOT BE PAINTED. BEARINGS SHALL BE FIXED AT THE PIER AND DESIGNED TO ALLOW MOVEMENT UNDER THERMAL EXPANSION OR CONTRACTION AT THE ABUITMENTS.

THE FOLLOWING ASSUMED UNFACTORED BEARING REACTIONS WERE USED FOR THE DESIGN OF THE ABUTMENTS AND PIER:

R.A.: DEAD LOAD - 66.4 KIPS	F.A.: DEAD LOAD - 13.0 KIPS
LIVE LOAD - 11.1 KIPS	LIVE LOAD - 11.0 KIPS
PEDESTRIAN - 36.4 KIPS	PEDESTRIAN - 7.0 KIPS
WIND (TRANS.) - 28.1 KIPS	WIND (TRANS.) - 6.3 KIPS
PIER: DEAD LOAD - 163.3 KIPS	

LIVE LOAD - 12.2 KIPS PEDESTRIAN - 90.1 KIPS WIND (TRANS.) - 66.8 KIPS

NAMEPLATE:

THE BRIDGE MANUFACTURE SHALL SECURE A NAMEPLATE TO THE STRUCTURE WITH THE MANUFACTURER'S NAME, MAXIMUM LOAD LIMITS AND SERIAL NUMBER.

FINISH/COATING SYSTEM:

A COMPLETE SHOP APPLIED 3-COAT IZEU PAINT SYSTEM ACCORDING TO C&MS 514 SHALL BE APPLIED TO ALL EXPOSED SURFACES OF THE STEEL TRUSSES. FIELD TOUCH-UP SHALL BE PERFORMED ACCORDING TO C&MS 514.17.C. THE FINISH COAT COLOR SHALL BE RED SAE AMS-STD-595 #11350 FOR THE END VERTICALS, EXTENDED TOP CHORDS, CANOPY BRACES, AESTHETIC END DIAGONALS AND END TOP BRACES. FINISH COAT COLOR SHALL BE GRAY SAE AMS-STD-595 #16515 FOR ALL OTHER TRUSS MEMBERS. ALL COSTS ASSOCIATED WITH FIELD TOUCH-UPS SHALL BE CONSIDERED INCIDENTAL AND SHALL BE INCLUDED WITH THIS ITEM.

SUBMITTALS:

SUBMIT SHOP DRAWINGS AND STRUCTURAL DESIGN CALCULATIONS FOR THE STEEL STRUCTURE, BEARINGS AND THE ASSOCIATED REINFORCED CONCRETE DECK SLAB ACCORDING TO C&MS 501.04, 501.05 AND 513.06.

THE BRIDGE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND STRUCTURAL CALCULATIONS TO THE OHIO DEPARTMENT OF TRANSPORTATION (BRIDGE OWNER) AND CONTRACTOR FOR ACCEPTANCE PRIOR TO BEGINNING FABRICATION.

SHOP DRAWINGS SHALL BE UNIQUE DRAWINGS PREPARED TO ILLUSTRATE THE SPECIFIC PORTION OF THE WORK TO BE DONE. ALL RELATIVE DESIGN INFORMATION INCLUDING BUT NOT LIMITED TO GOVERNING CODES, DESIGN PARAMETERS, MEMBER SIZES, MATERIAL PROPERTIES, BRIDGE REACTIONS, SHOP AND FIELD CONNECTION DETAILS, DECK DETAILS, DIMENSIONS RELATED TO SUBSTRUCTURES AND GENERAL NOTES SHALL BE CLEARLY SPECIFIED ON THE DRAWINGS. SHOP DRAWINGS SHALL BE ACCURATELY PREPARED BY SKILLED DRAFTERS TO BE COMPLETE IN EVERY RESPECT. DRAWINGS SHALL HAVE CROSS-REFERENCED DETAILS AND SHEET NUMBERS.

THE OWNER MUST PROVIDE A WRITTEN ACCEPTANCE LETTER OF SHOP DRAWINGS TO CONFIRM TYPE, STYLE AND GENERAL APPEARANCE OF PREFABRICATED STRUCTURE IN ACCORDANCE WITH CONTRACT DOCUMENTS.

WRITTEN ACCEPTANCE FROM BOTH CONTRACTOR AND OWNER MUST BE PROVIDED PRIOR TO INITIATING FABRICATION MILL TEST REPORTS:

CONTRACTOR MUST PROVIDE WRITTEN ACCEPTANCE OF MILL TEST REPORTS FROM SUPPLIER SHOWING COMPLIANCE WITH C&MS 711.01.

DELIVERY AND ERECTION:

THE CONTRACTOR SHALL COORDINATE WITH THE BRIDGE MANUFACTURER, THE DEPARTMENT AND OVERHEAD UTILITY OWNERS REGARDING THE DELIVERY AND ERECTION SCHEDULE. HAULING PERMITS AND FREIGHT CHARGES SHALL BE THE RESPONSIBILITY OF THE MANUFACTURER.

DELIVERY TO THE JOB SITE WILL BE BY TRUCKS BY MEANS OF GOOD HAUL ROADS UNLESS SPECIFIED OTHERWISE. THE BRIDGE MANUFACTURER SHALL PROVIDE DETAILED, WRITTEN INSTRUCTION PROCEDURES FOR PROPER LIFTING AND SPLICING OF BRIDGE COMPONENTS. THE CONTRACTOR SHALL PROVIDE A DETAILED WRITTEN ERECTION PLAN TO THE ENGINEER. THE CONTRACTOR SHALL BE REQUIRED TO VERIFY THAT THE PROPOSED SUBSTRUCTURE DIMENSIONS, WIDTHS AND ELEVATIONS WILL ACCOMMODATE THE PROPOSED PREFABRICATED BRIDGE, AND ADJUST ACCORDINGLY IF NEEDED. ANY ADJUSTMENTS SHALL BE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION, UNLESS SPECIFICALLY STATED OTHERWISE IN THESE PLANS.

THE CONTRACTOR SHALL COORDINATE WITH THE BRIDGE FABRICATOR TO OBTAIN ADEQUATE EQUIPMENT TO ERECT, LIFT AND INSTALL THE BRIDGE SPANS. THIS INCLUDES SPREADER BEAMS ETC. AS REQUIRED TO ENSURE THAT NO PORTION OF THE BRIDGE IS OVERSTRESSED DURING INSTALLATION OF THE STRUCTURE.

THE CONTRACTOR SHALL COORDINATE WITH THE DEPARTMENT AND LOCAL LAW ENFORCEMENT REGARDING ERECTION OF THE BRIDGE SPANS OVER S.R. 8. ERECTION OVER THE HIGHWAY SHALL OCCUR AT NIGHT ONLY BETWEEN THE HOURS OF 9:00 PM TO 5:00 AM TO ENSURE MINIMAL DISTURBANCE TO THE TRAVELING PUBLIC. ADVANCED WARNING SIGNAGE SHALL ALERT THE PUBLIC OF TEMPORARY NIGHTLY LANE CLOSURES AT LEAST FOURTEEN DAYS PRIOR TO THE START OF CONSTRUCTION.

THE BRIDGE MANUFACTURER SHALL PROVIDE WRITTEN INSPECTION AND MAINTENANCE PROCEDURES TO BE FOLLOWED TO THE BRIDGE OWNER.

WARRANTY:

THE BRIDGE MANUFACTURER SHALL PROVIDE THE BRIDGE OWNER WITH A WRITTEN WARRANTY AGAINST DEFECTS IN DESIGN, MATERIAL AND WORKMANSHIP OF THE PREFABRICATED BRIDGE SUPERSTRUCTURE FOR A PERIOD OF TEN YEARS FROM THE DATE OF DELIVERY TO THE SITE. PAINT AND OTHER SPECIAL COATINGS SHALL BE WARRANTED BY THE COATING MANUFACTURER. REPAIR OR REPLACEMENT OF THE SUPERSTRUCTURE BY THE MANUFACTURER SHALL BE THE SPECIFIC REMEDY FOR DEFECTS UNDER THE WARRANTY. AS PART OF THE WARRANTY COVERAGE, THE BRIDGE OWNER WILL KEEP RECORDS OF ROUTINE INSPECTIONS AND MAINTENANCE OF THE BRIDGE. YEARLY BRIDGE INSPECTIONS WILL BE PERFORMED BY THE OWNER.

PAYMENT:

PAYMENT FOR THE PREFABRICATED/PAINTED STEEL SUPERSTRUCTURE SHALL BE MADE AT A LUMP SUM BID PRICE AND SHALL INCLUDE ALL ITEMS LISTED ABOVE AND SHOWN ON THE PLANS, MATERIALS, LABOR, EQUIPMENT AND INCIDENTALS NECESSARY TO COMPLETE THE REQUIRED WORK. NOTE THAT THE DESIGN OF THE DECK AND THE DESIGN AND MATERIAL FOR THE STAY-IN-PLACE FORM SYSTEM SHALL BE PROVIDED BY THE BRIDGE MANUFACTURER AND ARE INCLUDED IN ANOTHER PAY ITEM.

THE OWNER SHALL NOT BE RESPONSIBLE FOR ADDED EXPENSE DUE TO UNAVOIDABLE DELAYS SUCH AS INCLEMENT WEATHER, DELAYS IN PERMITS, RE-ROUTING BY PUBLIC AGENCIES, ETC.

<u>ITEM</u>	<u>UNIT</u>	<u>DESCRIPTION</u>
SPECIAL	LUMP	STRUCTURE MISC.: PREFABRICATED PAINTED STEEL SUPERSTRUCTURE

ITEM SPECIAL - STRUCTURE MISC.: CLASS QC2 CONCRETE BRIDGE DECK:

THIS WORK SHALL CONSIST OF THE COMPLETE STRUCTURAL DESIGN OF THE DECK AND FORM SYSTEM BY THE PREFABRICATED BRIDGE DESIGNER AND MANUFACTURER. THE WORK SHALL INCLUDE BUT NOT BE LIMITED TO: PROVIDING HOT DIP GALVANIZED STAY-IN-PLACE FORM PANS (MATERIAL SHALL BE IN ACCORDANCE WITH ASTM A653, GALVANIZED TO A MINIMUM GI65 COATING WEIGHT, 20-GAUGE MINIMUM THICKNESS) AND ALL OTHER NECESSARY FORMING; FURNISHING AND PLACING EPOXY COATED REINFORCING STEL; AND FURNISHING, PLACING, CONSOLIDATING, FINISHING AND CURING A PORTLAND CEMENT CONCRETE DECK SLAB WITH INTEGRAL TYPE 2-A CURBS ON THE PREFABRICATED PAINTED STEEL TRUSS SUPERSTRUCTURE. ALL WORK AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF THE PREFABRICATED BRIDGE MANUFACTURER AND C&MS 508, 509 AND 511, UNLESS OTHERWISE NOTED. FOR THE PURPOSES OF DECK SLAB STRUCTURAL DESIGN, THE GALVANIZED STAY-IN-PLACE FORM SYSTEM SHALL NOT BE CONSIDERED AS A STRUCTURAL REINFORCEMENT OF THE HARDENED CONCRETE DECK. FOAM OR OTHER FILLERS WITHIN THE TROUGHS OF THE FORM PANS WILL NOT BE PERMITTED.

CONCRETE MATERIALS SHALL CONFORM TO C&MS 499.02 AND 499.03 USING CLASS QC2 CONCRETE MIX. PROVIDE A BROOM FINISH ON THE CONCRETE DECK IN THE TRANSVERSE DIRECTION. BRIDGE DECK GROOVING OF THE CURED DECK PER C&MS 511.17 IS NOT REQUIRED.

DESIGN LOADING FOR THE DECK SLAB SHALL BE THE SAME AS REQUIRED FOR THE PREFABRICATED BRIDGE. UPPER AND LOWER LAYERS OF LONGITUDINAL REINFORCEMENT ARE REQUIRED. AT LEAST ONE LAYER OF TRANSVERSE REINFORCEMENT SHALL BE PROVIDED WHEN THE DECK THICKNESS ABOVE THE FORM PAN RIBS IS LESS THAN 7 1/2-INCHES. UPPER AND LOWER LAYERS OF TRANSVERSE REINFORCEMENT SHALL BE PROVIDED WHEN THE DECK THICKNESS ABOVE THE FORM PAN RIBS IS 7 1/2-INCHES OR GREATER. LONGITUDINAL REINFORCEMENT PLACED WITHIN FORM PAN TROUGHS MAY BE CONSIDERED AS CONTRIBUTING TO THE STRENGTH OF THE DECK WHEN THE DESIGNER CAN SHOW THIS ASSUMPTION IS VALID. REINFORCING BARS SHALL BE PLACED 2-INCHES MINIMUM CLEAR TO TOP AND SIDE SURFACES AND 1 1/2-INCHES MINIMUM CLEAR TO THE BOTTOM SURFACE OF THE SLAB. AS PART OF THE PREFABRICATED BRIDGE SHOP DRAWING SUBMITTAL, SUBMIT STRUCTURAL DESIGN CALCULATIONS FOR THE DECK AND FORM SYSTEM. CALCULATIONS MUST BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF OHIO.

THE I FOR:	DEPARTM	ENT WILL PAY	FOR THE DE	СК	Released for Construction	on		
<u>ITEM</u> SPEC	IAL	<u>UNIT</u> LUMP			10/06/2021 <u>IPTION</u> MISC.: CLASS QC2		C۲	re 300
ITEM	SPECIAL	- FORM LINE		ΤE	BRIDGE DECK		N AGEN	43240 Sui
ON T #911C THE I OTHE SIDE	HE PLANS , LARGE ENGINEER R PARTS OF NOIS	5. FORM LINE STONE OHIO . THE STONE OF THE PROJ	ERS SHALL BE DRY STACK C FORM LINER JECT (RETAIN. ENSURE UNIFO	TA DR P IN(OR	ARD ABUTMENTS AS SHOW RCHITECTURAL POLYMERS EQUAL AS APPROVED BY ATTERN SHALL MATCH S WALLS AND ROADWAY M SURFACE TREATMENTS MEER.		DESIGN	B415 Pulsar Plo Columbus Ohio
CONC PHYS CAUS PROD USE MANU BE FO EPOX	RETE PO ICAL DEF ING CON(DUCT AS P MANUFAC IFACTURE ULLY COM	UR PRESSURES ECTS. FORM CRETE SURFAC RECOMMENDED TURER'S APPL. R'S INSTRUCT. MPATIBLE WITH	S WITHOUT LE LINERS SHAL CE DAMAGE. D BY THE FOR ICATION RATIONS. FORM H THE FORM L	EAP L US M ES RE LIN	STANDING ANTICIPATED KAGE OR CAUSING BE REMOVABLE WITHOUT E A FORM RELEASE LINER MANUFACTURER. AND ALL OTHER ELEASE PRODUCTS SHALL VER MATERIAL AND THE TO THE FINISHED		REVIEWED DATE SAN 6.711731	JRE F1 702
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- FO LABC COMF	RM LINER DR, MATEH PLETE TH.	, WHICH SHALI RIALS, EQUIPN	L BE FULL CO MENT, AND IN PECIFIED ABO	ЭМ. ICI	EE BID FOR ITEM SPECIAL PENSATION FOR ALL DENTALS NECESSARY TO E AND IN A SATISFACTOR)			
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OR	OHIO EP P.O. BO COLUMBL	X 1049 JS, OH 43216- DS PROGRAM	-1049			KFC PLANS		
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NECE FORN	SSARY TO 1. PAYMEI	O COMPLETE A NT FOR THIS I	AND SUBMIT 1 WORK SHALL	THE BE	S, LABOR, AND MATERIAL E OEPA NOTIFICATION E INCLUDED IN ITEM 202 N, AS PER PLAN.		4	5 17

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			ESTIMATED QUANTITIES				CALC.	DATE	CHECKED	DA/96/2021	
			ESTIMATED QUANTITIES				CAS	08/21/2019	ODW	8/29/2019	
ITEM	ITEM EXT.	UNIT	DESCRIPTION	ABUTMENTS	PIERS S	SUPERSTR.	GENERAL	TOTAL	PARTICIPATION 11/IMS/BR	- SHEET REF.	
			OPTION A								
202	11003	LS	STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN				050	LS	LS	6 / 16	
202	22900	SY	APPROACH SLAB REMOVED				250	250	250		
503	21100	CY	UNCLASSIFIED EXCAVATION	168				168	168		
503	31100	CY	ROCK EXCAVATION	44				44	44	_	
503	31120	СҮ	SHALE EXCAVATION	24				24	24		
509	10000	LB	EPOXY COATED REINFORCING STEEL	8474	8398			16872	16872		
511	41010	СҮ	CLASS QC1 CONCRETE, PIER ABOVE FOOTINGS		15			15	15		
511	43510	СҮ	CLASS QC1 CONCRETE, ABUTMENT INCLUDING FOOTING	92				92	92		
512	10100	SY	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	69	50			119	119		
512	33000	SY	TYPE 2 WATERPROOFING	24				24	24		
516	10001	FT	PREFORMED ELASTOMERIC COMPRESSION JOINT SEAL, AS PER PLAN	26				26	26	3 / 16	
516	13200	SF	1/2" PREFORMED EXPANSION JOINT FILLER	101				101	101		
516	13600	SF	1" PREFORMED EXPANSION JOINT FILLER	45				45	45		
518	21200	СҮ	POROUS BACKFILL WITH GEOTEXTILE FABRIC	44				44	44		
518	40000	FT	6" PERFORATED CORRUGATED PLASTIC PIPE	34				34	34		
524	94907	FT	DRILLED SHAFTS, 54" DIAMETER, ABOVE BEDROCK, AS PER PLAN		10			10	10	3 / 16	
524	94909	FT	DRILLED SHAFTS, 54" DIAMETER, INTO BEDROCK, AS PER PLAN		17			17	17	3 / 16	
524	95100	EACH	DRILLED SHAFT, MISC.: THERMAL INTEGRITY PROFILER (T.I.P.) WIRE CABLE TESTING OF DRILLED SHAFTS		1			1	1		
SPECIAL	53000200	LS	STRUCTURE, MISC.: PREFABRICATED PAINTED STEEL SUPERSTRUCTURE					LS	LS	3 - 4 / 16	
SPECIAL	53000200	LS	STRUCTURE. MISC.: CLASS QC2 CONCRETE BRIDGE DECK					LS	LS	4 / 16	
SPECIAL	53013000	SF	FORM LINER	341				341	341	4 / 16	
SPECIAL	69098400	LS	MISC.: WORK INVOLVING ASBESTOS CONTAINING MATERIALS					LS	LS	4 / 16	
										1	
			OPTION B: ATC								
SPECIAL	20299000	LS	STRUCTURE REMOVED				LS	LS	LS		
SPECIAL	51299000	LS	SEALING OF CONCRETE	LS	LS			LS	LS		
SPECIAL	53099010	LS	SUBSTRUCTURE	LS	LS			LS	LS		
SPECIAL	53099020	LS	SUPERSTRUCTURE			LS		LS	LS		

STANDARD ABBREVIATIONS LIST:

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DESCRIPTION

ISSUE RECORD: NO. DATE

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BOT. = BOTTOM BRG. = BEARINGS BRGS. = BEARINGS B.S. = BOTH SIDES C/C = CENTER-TO-CENTER C.J. = CONSTRUCTION JOINT CJP = COMPLETE JOINT PENETRATION CLR. = CLEAR C&MS = CONSTRUCTION AND MATERIALS SPECIFICATIONS CONST. = CONSTRUCTION AND MATERIALS SPECIFICATIONS CONST. = CONSTRUCTION DIA. = DIAMETER E.F. = EACH FACE EL. = ELEVATION EMBED. = EMBEDMENT EO. = EQUAL ENDED: - EMBEDMENT EQ. = EQUAL EXP. = EXPANSIONF.A. = FORWARD ABUTMENT f/f = FACE-TO-FACEF.F. = FAR FACE EVEN F.F. = FAR FACE FWD = FORWARD LT. = LEFT MAX. = MAXIMUM M.E. = MATCH EXISTING MIN. = MINIMUM NB = NORTHBOUND N.F. = NEAR FACE N.F. = NEAR FACE NPCPP = NON-PERFORATED CORRUGATED PLASTIC PIPE 0/0 = OUT-TO-OUT PCB = PORTABLE CONCRETE BARRIER PCPP = PERFORATED CORRUGATED PLASTIC PIPE PEJF = PREFORMED EXPANSION JOINT FILLER P.G. = PROFILE GRADE

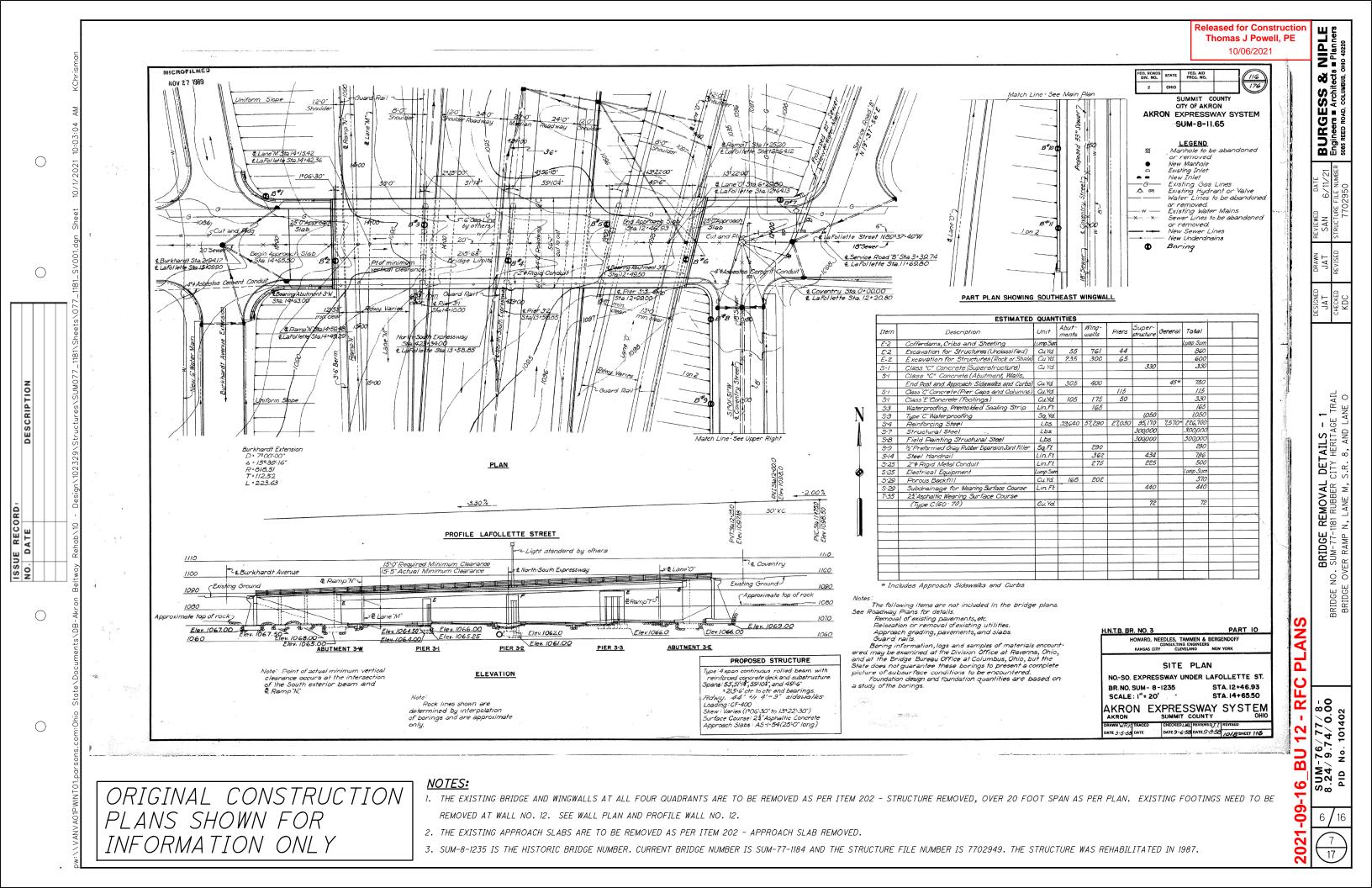
R.A. = REAR ABUTMENT RAD. = RADIUS RT. = RIGHT SB = SOUTHBOUND SHLD. = SHOULDER S.O. = SERIES OF SPA. = SPACES STA. = STATION SYMM. = SYMMETRICAL T&B = TOP AND BOTTOM T/R = TOP OF ROCK t/t = TOE-TO-TOE U.N.O. = UNLESS NOTED C U.N.O. = UNLESS NOTED OTHERWISE

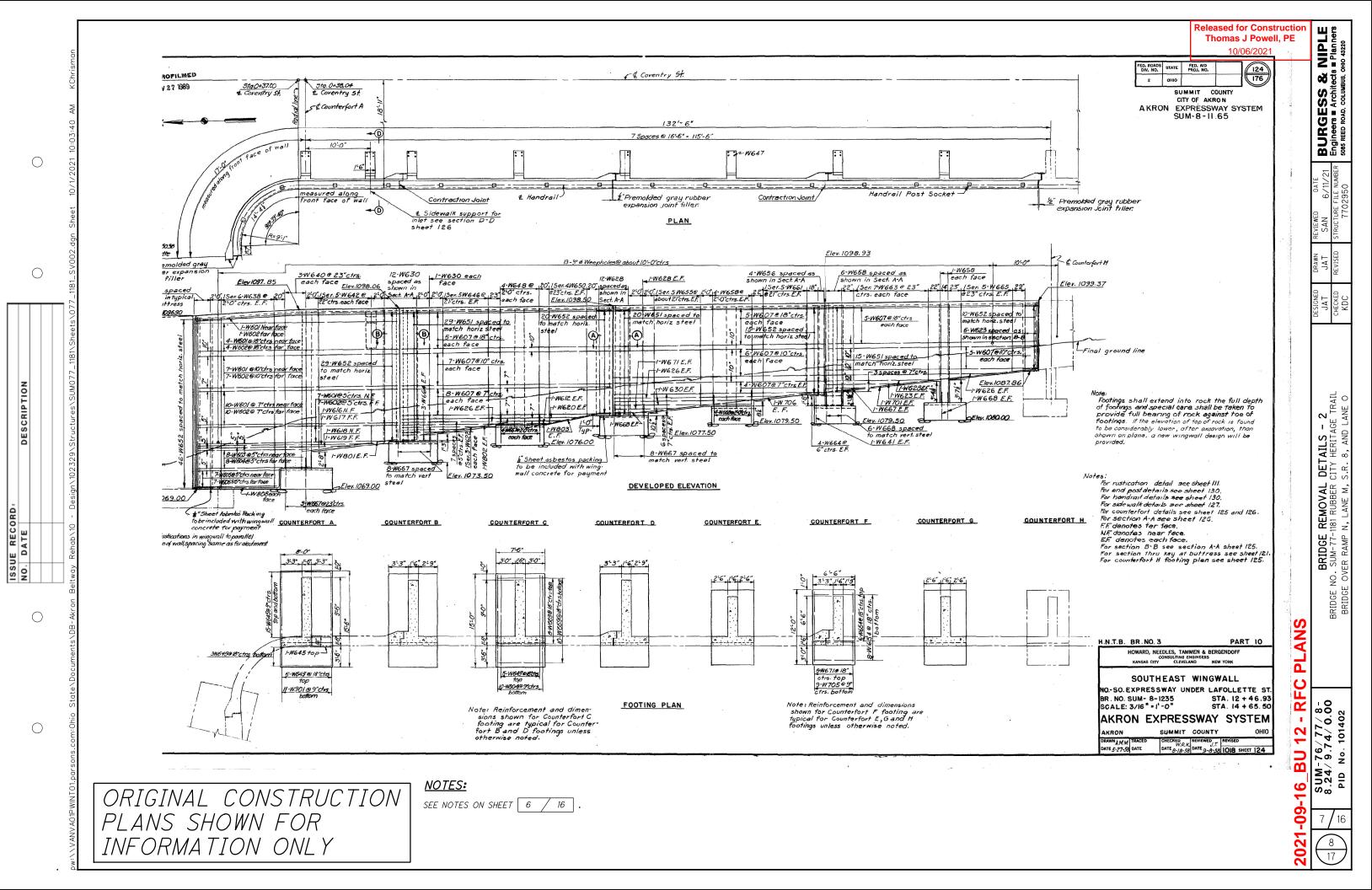
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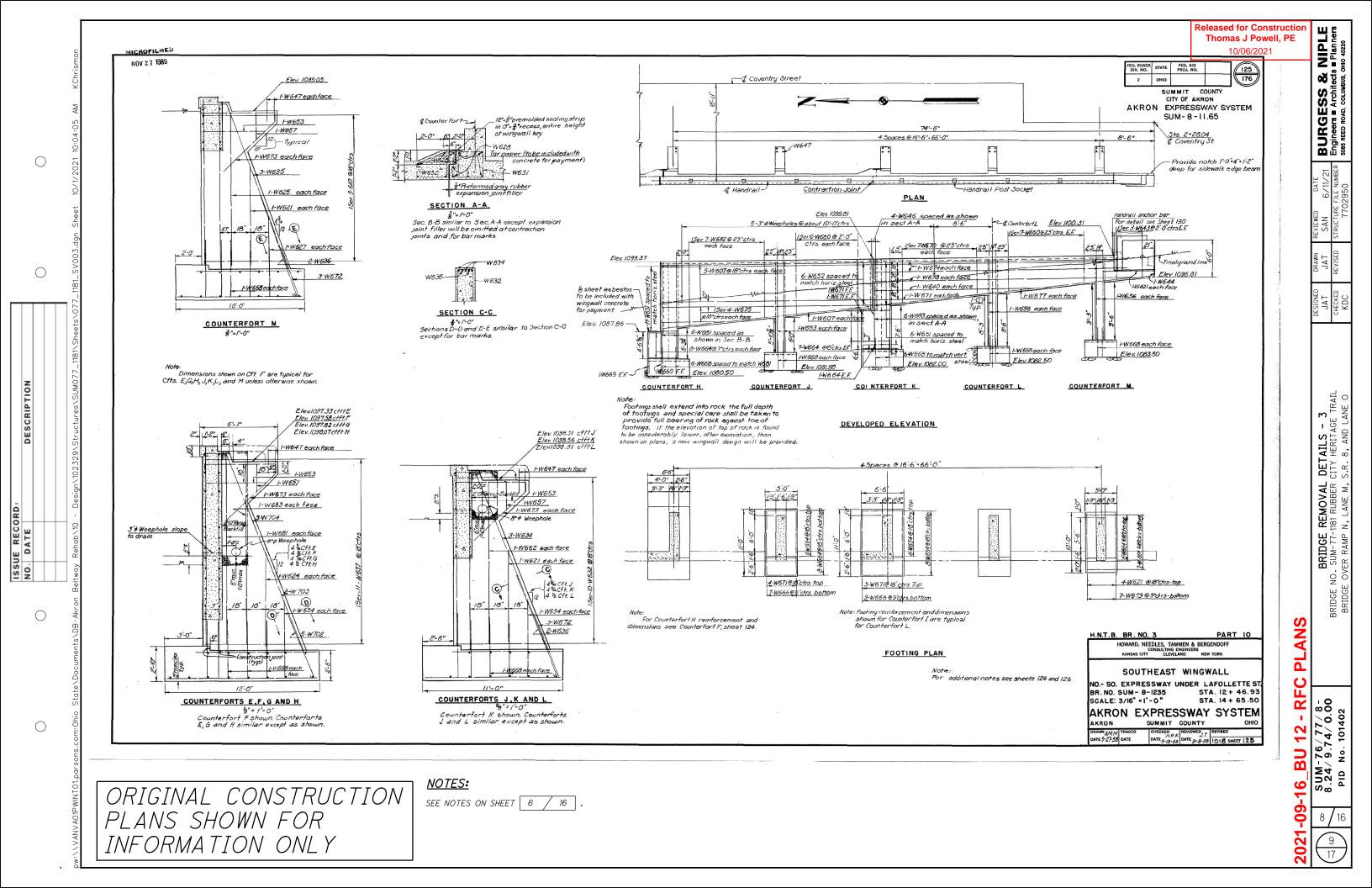
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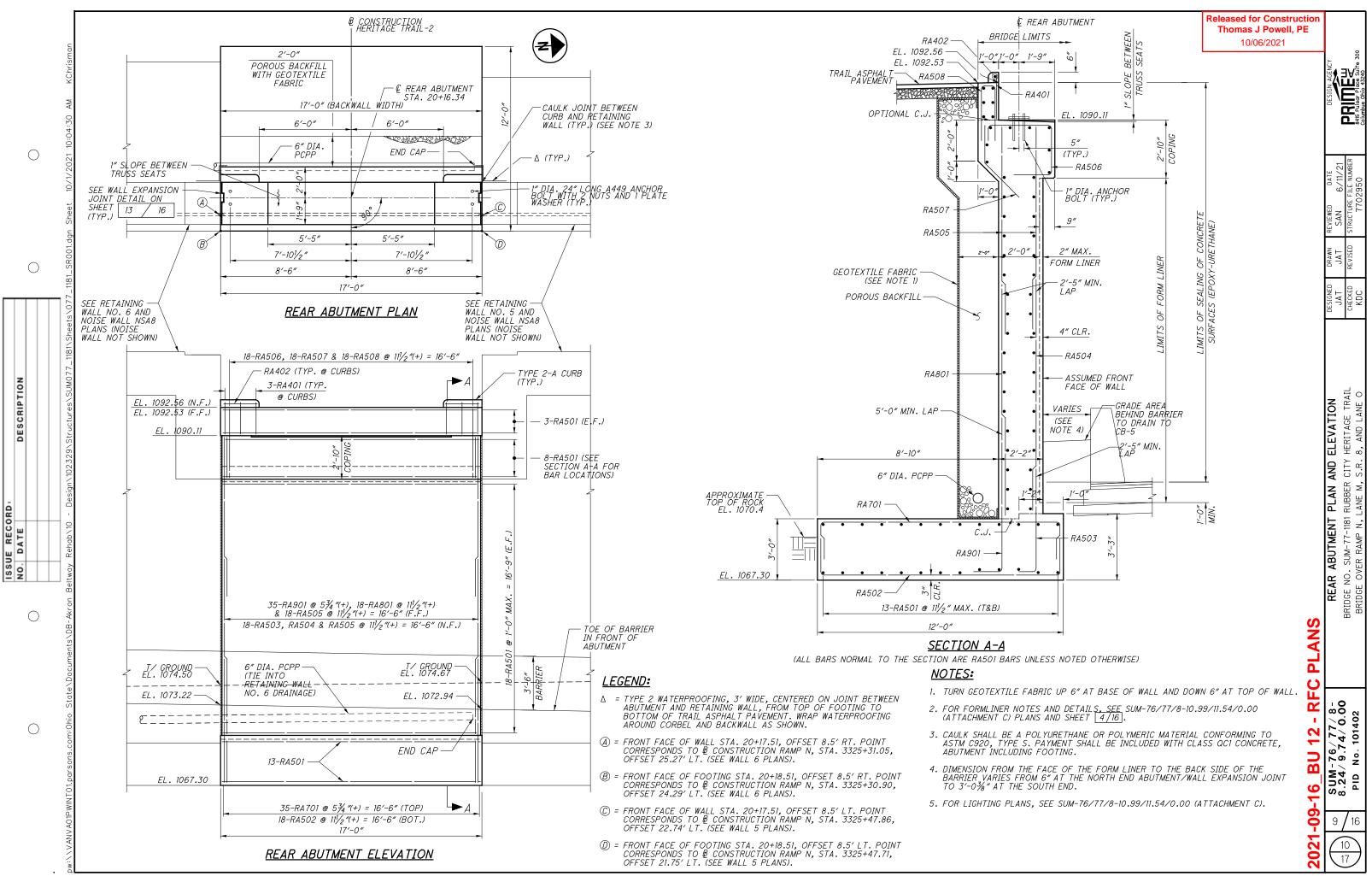
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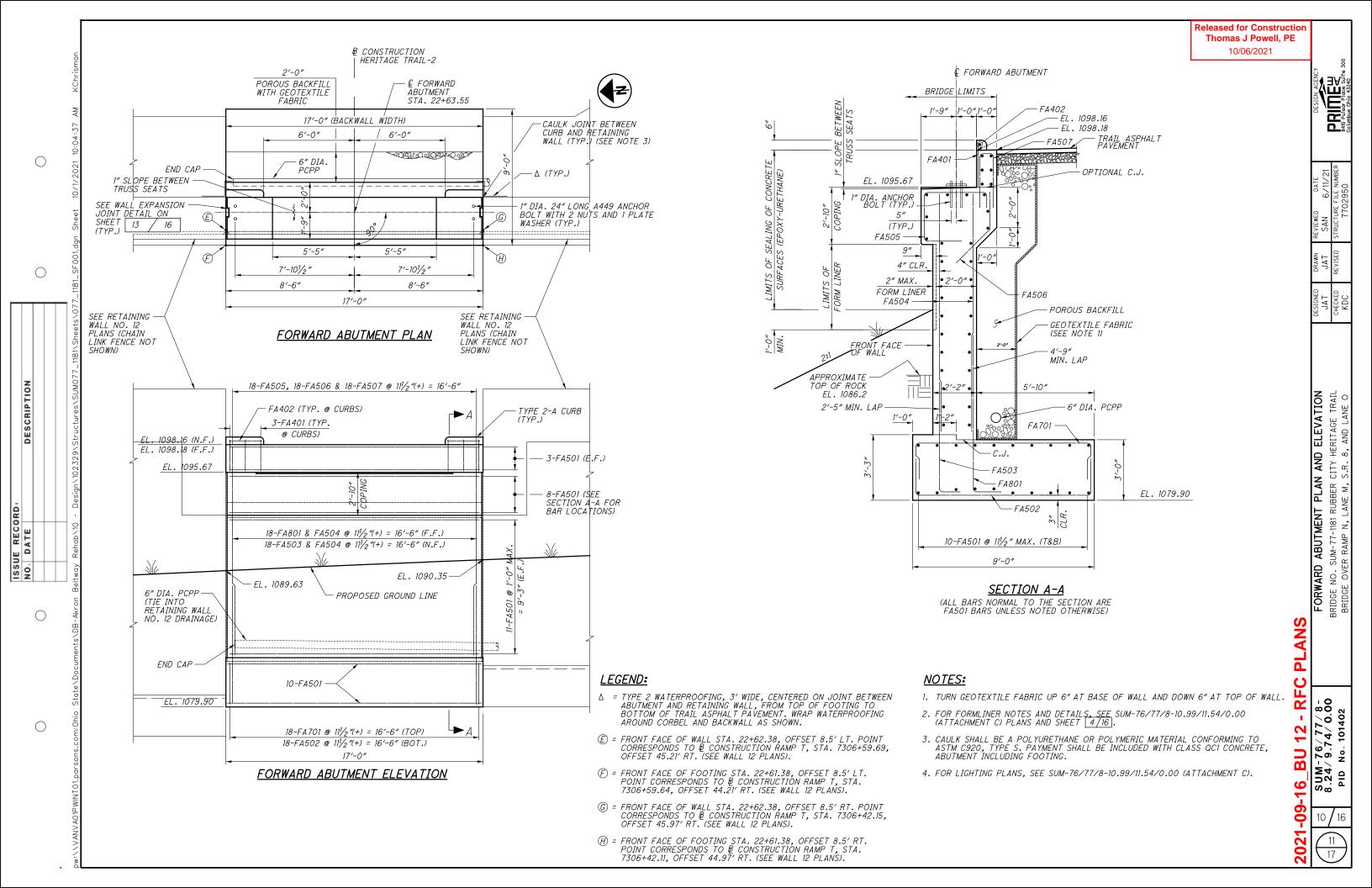
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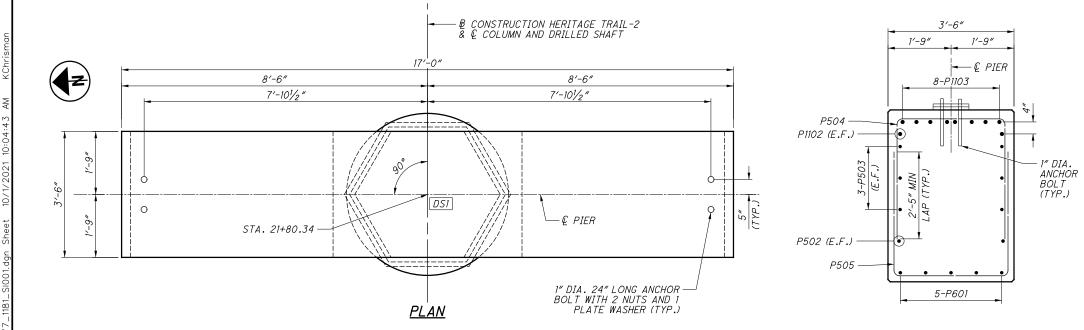












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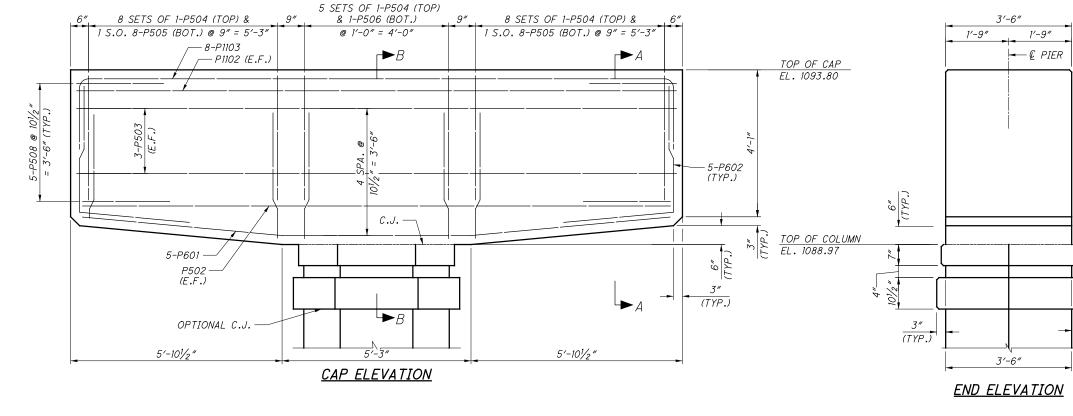
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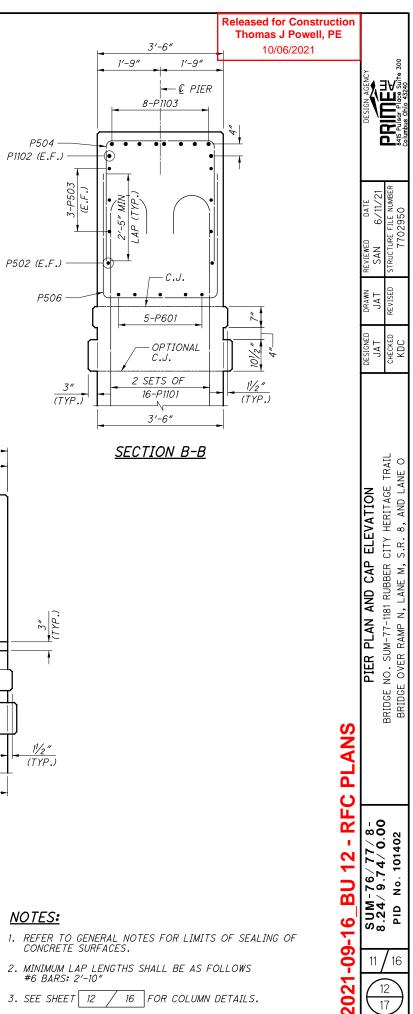
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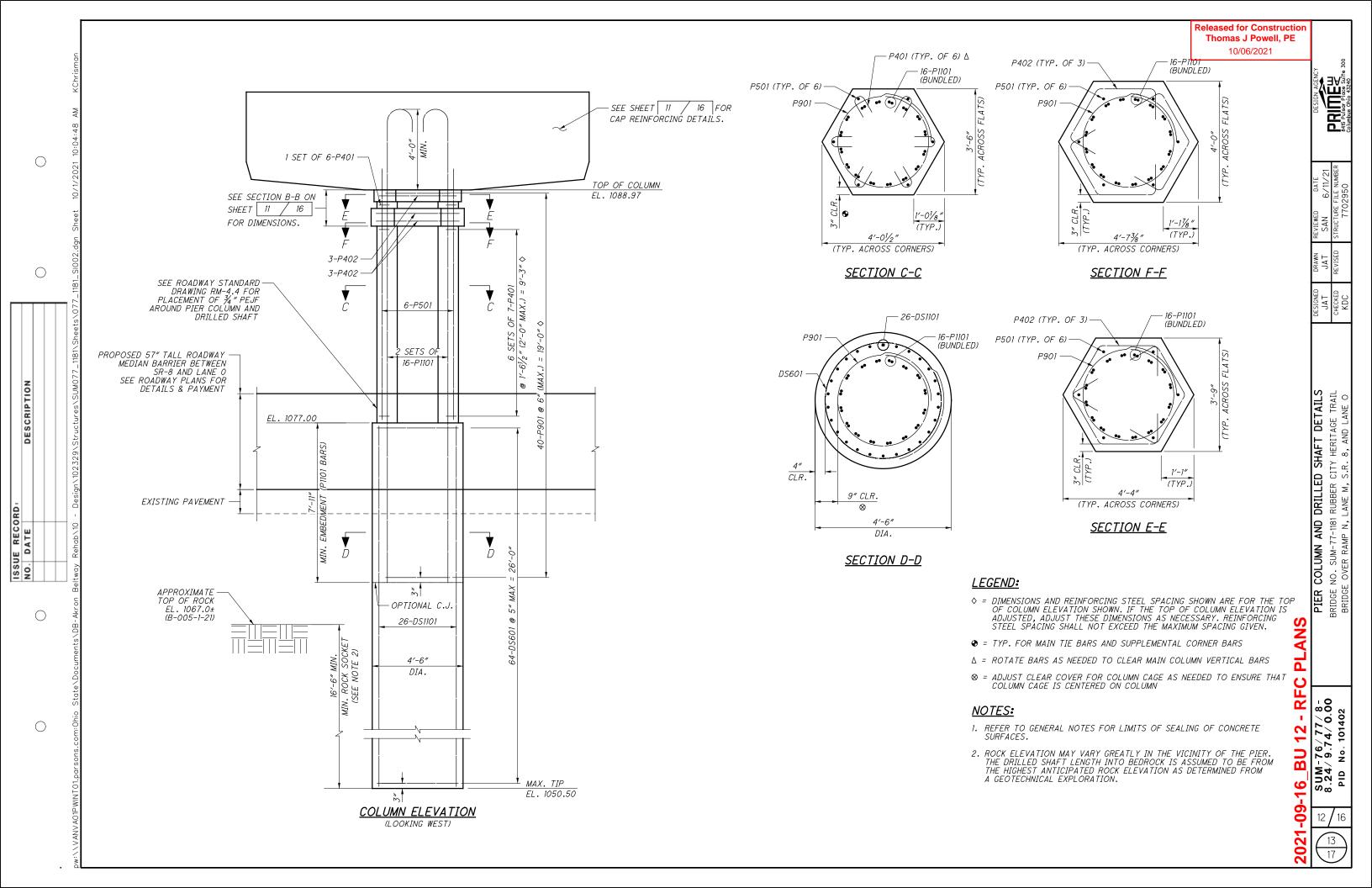
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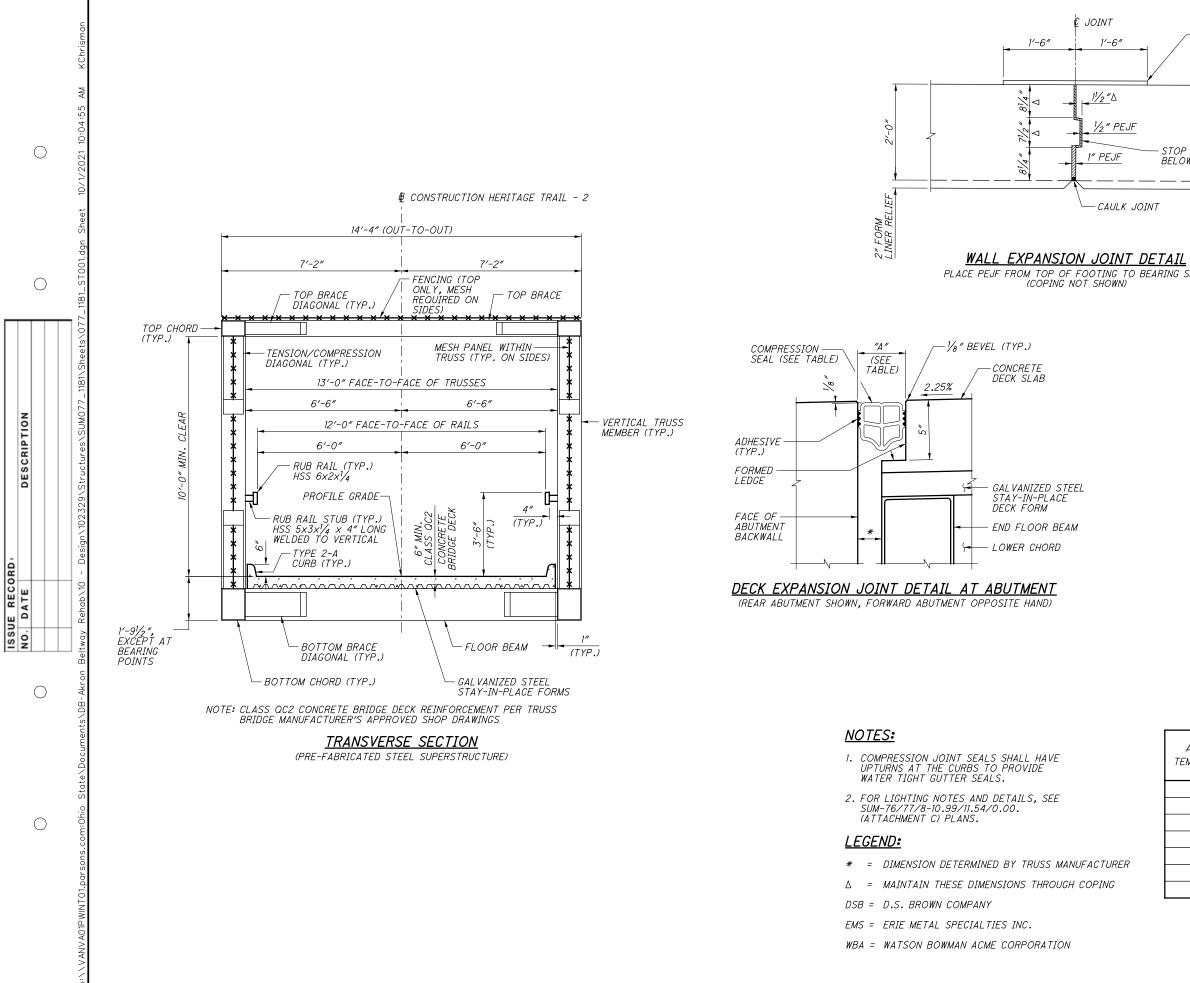
SECTION A-A



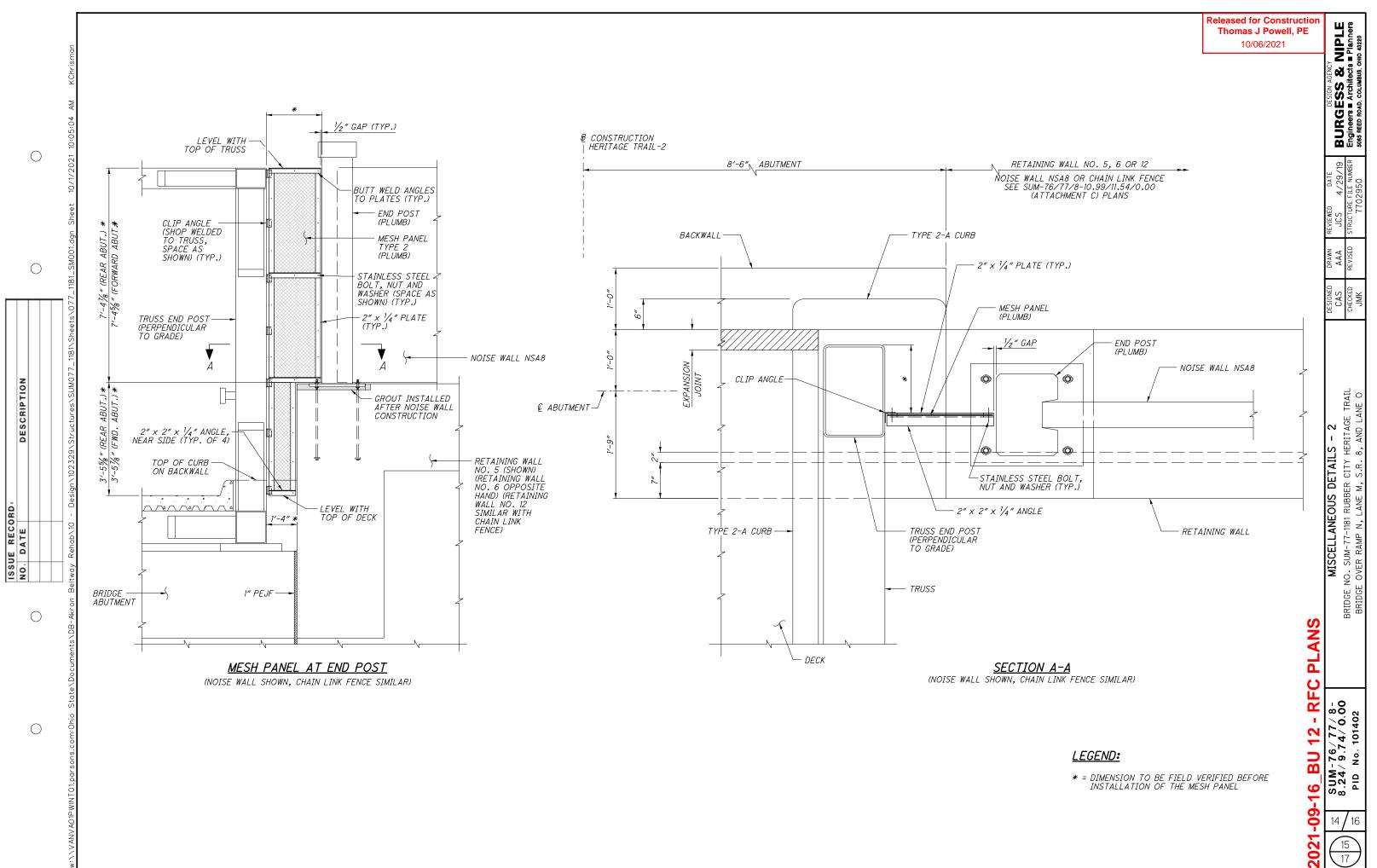
LEGEND: DS1 = DRILLED SHAFT NUMBER



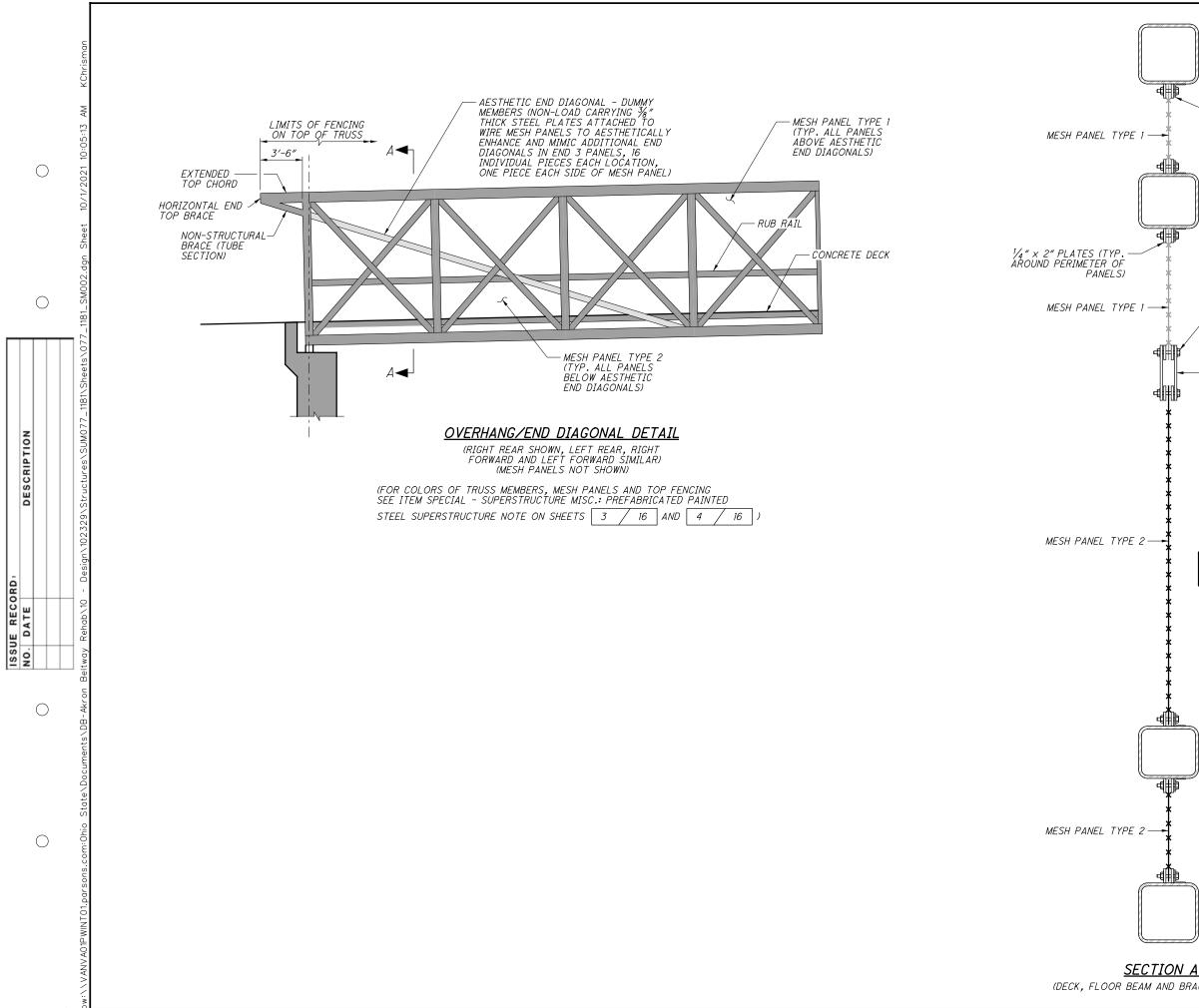




			Thomas	or Construction J Powell, PE 06/2021		
	E TYPE 2 DOFING. TOP OF TO 2½″ BELOW GRADE. (BACKW2 W)	- 4 <i>LL</i>			DESIGN AGENCY	
D KEY 6″ DW TRUSS SEA ────	Τ				REVIEWED DATE SAN 6/11/21	STRUCTURE FILE NUMBER 7702950
SEAT					DRAWN JAT	REVISED
					DESIGNED JAT	CHECKED KDC
				09-16_BU 12 - RFC PLANS	MISCELLANEOUS DETAILS - 1	BRIDGE NO. SUM-77-1181 RUBBER CITY HERITAGE TRAIL BRIDGE OVER RAMP N, LANE M, S.R. 8, AND LANE O
AMBIENT EMPERATURE		DIMENSION "A" ACTURER/SEAL	TYPE	_		
	DSB/JP-350	WBA/100FW	EMS/JP-400	l Ö	┣—	
30°F	311/16"	43/16"	43/16"		<u></u>	2
40°F	3%6″	4 ¹ / ₁₆ "	4 ¹ / ₁₆ "	-	- 8 / 2 00 00	102
50°F	3 ¹ /2 "	315/ "	3 ¹⁵ /16″	<u>2</u>		017
	33%"	3 ¹³ /6″ 3 ¹¹ /6″	3 ¹³ /6″ 3 ¹¹ /6″		76/7	ς Τ.
60°F	1///	11/40") <i>'//e″</i>			° ć
60°F 70°F 80°F	<u> </u>		39/16"	.	SUM-7	PID No. 101402



DESCRIPTION



	Released for Construction Thomas J Powell, PE	
TOP CHORD	10/06/2021	SENCY & NIF ecta = Pla 308, OHIO 433
CLIP ANGLE WELDED TO TRUSS MEMBER (TYP.)		DESIGN AGENCY BURGESS & NIPLE Engineers = Architects = Planners 5085 RED ROAD, COLUMBUR, OHIO 4320
DIAGONAL		REVIEWED DATE JCS 4/29/19 JCS 4/29/19 STRUCTURE FILE NUMBER 7702950
— STAINLESS STEEL BOLT (TYP.)		DRAWN AAA REVISED
/ AESTHETIC END DIAGONAL DUMMY MEMBERS (¾ " THICK PLATES)		DESIGNED CAS CHECKED JMK
DIAGONAL	LANS	MISCELLANEOUS DETAILS - 3 BRIDGE NO. SUM-77-1181 RUBBER CITY HERITAGE TRAIL BRIDGE OVER RAMP N, LANE M, S.R. 8, AND LANE O
BOTTOM CHORD	2021-09-16_BU 12 - RFC PLANS	SUM-76/77/8- 8.24/9.74/0.00 PID No.101402
<u>A-A</u> RACING NOT SHOWN	2021-09-16	$\begin{array}{c} \mathbf{S}_{\mathbf{\infty}} \\ 15 \\ 16 \\ 17 \\ 17 \\ 17 \\ 17 \\ 17 \\ 16 \\ 17 \\ 17$

MARK	NO.	LENGTH	WEIGHT	TYPE	DIM. A	DIM. B	DIM. C	DIM. D	DIM. E/RAD.	INCR.	
RA401	6	1'-4″	5	STR							
RA402	2	2'-0"	3	STR							
RA501	76	16'-6″	1308	STR							
RA502	18	15′-1″	283	18	11′-8″	1'-10″	1′-10″				
RA503	18	6'-4"	119	1	0'-10"	5′-7″					
RA504	18	11'-3"	211	STR							
RA505 RA506	36 18	10'-5" 8'-3"	391 155	STR 25	1′-10″	2'-6"	3′-5″				A
RA506 RA507	18	8'-3" 5'-3"	99	25	1'-10"	2'-6"	2'-4"				
RA507 RA508	18	5-5 7'-1″	133	18	0'-8"	3'-4"	2 -4 3'-4"				
RA 701	35	15'-0"	1073	18	11'-8″	1'-10"	1'-10"				TYPE
RA801	18	11'-6″	553	STR	11 0	1 10	1 10				<u>, ,,, c</u>
RA901	35	9'-3"	1101	1	1'-7″	7'-11″					
		TOTAL	5434								
MARK FA401	NO. 6	LENGTH 1'-4"	WEIGHT 5	TYPE STR			RCING S DIM. C	STEEL LI DIM. D	ST DIM. E/RAD.	INCR.	
FA402	2	2'-0"	3	STR							
FA501	56	16'-6"	964	STR							
FA502	18	12'-1"	227	18	8'-8"	1'-10"	1′-10″				
FA503	18	6'-4"	119	1	0'-10"	5′-7″					A L
FA504	36	12'-3"	460	STR	1/ 10 //	0/ 0//	7/		+		
FA505 FA506	18 18	8'-3" 5'-3"	155 99	25 20	1′-10″ 1′-10″	2'-6" 2'-4"	3'-5" 2'-4"				
	18										/
				/×	()'_&"	3'_4"	3'_1"				
FA507 FA701		7'-1" 12'-0"	133 442	18 18	0′-8″ 8′-8″	3'-4"	3'-4"				
FA701	18	12'-0″	442	18	8'-8"	1'-10″	3'-4" 1'-10"				
											<u> </u>
FA701	18	12'-0" 9'-0"	442 433 3040 PIE	18 1	8'-8" 1'-4" EINFOR	1'-10" 7'-11"		IST			
FA701 FA801 MARK	18 18 	12'-0" 9'-0" TOTAL	442 433 3040 PIE WEIGHT	18 1 R RE TYPE	8'-8" 1'-4" INFOR DIM. A	1'-10" 7'-11" CING S DIM. B	1'-10"	IST DIM. D	DIM. E/RAD.	INCR.	<u> </u>
FA701 FA801 MARK P401	18 18 NO. 42	12'-0" 9'-0" TOTAL LENGTH 1'-6"	442 433 3040 PIE WEIGHT 42	18 1 R RE TYPE 21	8'-8" 1'-4" EINF OR DIM. A 4"	1'-10" 7'-11" CING S DIM. B 8"	1'-10" ТЕЕL L. DIM. С	DIM. D	DIM. E/RAD.	INCR.	<u> </u>
FA701 FA801 MARK P401 P402	18 18 NO. 42 9	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8"	442 433 3040 PIE WEIGHT 42 34	18 1 R RE TYPE 21 41	8'-8" 1'-4" INFOR DIM. A	1'-10" 7'-11" CING S DIM. B	1'-10" TEEL L		DIM. E/RAD.	INCR.	<u>TYPE</u>
FA701 FA801 MARK P401 P402 P501	18 18 NO. 42 9 6	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8"	442 433 3040 PIE WEIGHT 42 34 79	18 1 R RE TYPE 21 41 STR	8'-8" 1'-4" EINF OR DIM. A 4"	1'-10" 7'-11" CING S DIM. B 8"	1'-10" ТЕЕL L. DIM. С	DIM. D	DIM. E/RAD.	INCR.	
FA701 FA801 MARK P401 P402 P501 P502	18 18 NO. 42 9 6 2	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4"	442 433 3040 PIE WEIGHT 42 34 79 34	18 1 <i>R RE</i> <i>TYPE</i> 21 41 <i>STR</i> <i>STR</i>	8'-8" 1'-4" EINF OR DIM. A 4"	1'-10" 7'-11" CING S DIM. B 8"	1'-10" ТЕЕL L. DIM. С	DIM. D	DIM. E/RAD.	INCR.	<u>TYPE</u>
FA701 FA801 MARK P401 P402 P501 P502 P503	18 18 NO. 42 9 6 2 6	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8"	442 433 3040 PIE WEIGHT 42 34 79 34 104	18 1 <i>R RE</i> <i>TYPE</i> 21 41 <i>STR</i> <i>STR</i> <i>STR</i>	8'-8" 1'-4" EINFOR DIM. A 4" 1'-0"	1'-10" 7'-11" CING S DIM. B 8" 1'-8"	1'-10" TEEL L. DIM. C 11"	DIM. D	DIM. E/RAD.	INCR.	
FA701 FA801 MARK P401 P402 P501 P502	18 18 NO. 42 9 6 2 6 21	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8" 9'-7"	442 433 3040 PIE WEIGHT 42 34 79 34	18 1 <i>R RE</i> <i>TYPE</i> 21 41 <i>STR</i> <i>STR</i>	8'-8" 1'-4" EINF OR DIM. A 4"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4"	1'-10" TEEL L. DIM. C 11" 3'-4"	DIM. D	DIM. E/RAD.	INCR.	
FA701 FA801 MARK P401 P402 P501 P502 P503	18 18 NO. 42 9 6 2 6	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8"	442 433 3040 PIE WEIGHT 42 34 79 34 104	18 1 <i>R RE</i> <i>TYPE</i> 21 41 <i>STR</i> <i>STR</i> <i>STR</i>	8'-8" 1'-4" EINFOR DIM. A 4" 1'-0"	1'-10" 7'-11" CING S DIM. B 8" 1'-8"	1'-10" TEEL L. DIM. C 11"	DIM. D	DIM. E/RAD.		
FA701 FA801 MARK P401 P402 P501 P502 P503 P504	18 18 NO. 42 9 6 2 6 21 2	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-7" 9'-7" 9'-1" TO	442 433 3040 PIE WEIGHT 42 34 79 34 104 210	18 1 TYPE 21 41 STR STR STR 18	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-1" TO	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-1" TO	DIM. D	DIM. E/RAD.	INCR.	
FA701 FA801 MARK P401 P402 P501 P502 P503 P504	18 18 18 18 18 18 18 42 6 2 6 21 2 5.0.	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8" 9'-7" 9'-1"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210	18 1 TYPE 21 41 STR STR STR 18	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-1"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-1"	DIM. D	DIM. E/RAD.		
FA701 FA801 MARK P401 P501 P502 P503 P504 P505 P506	18 18 18 18 18 18 18 42 9 6 2 6 21 2 6 21 2 5.0. 8	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8" 9'-7" 9'-1" TO 10'-1" 10'-1"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160	18 1 TYPE 21 41 STR STR STR 18 18	8'-8" 1'-4" <u>I'-4"</u> <u>DIM. A</u> 4" 1'-0" <u>3'-2"</u> <u>3'-2"</u>	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-1" TO 3'-7"	1'-10" TEEL L DIM. C 11" 3'-4" 3'-1" TO 3'-7"	DIM. D	DIM. E/RAD.		
FA701 FA801 FA801 P401 P402 P501 P502 P503 P504 P505 P506 P507 P508	18 18 18 18 18 18 18 9 6 21 2 6 21 2 5.0. 8 5	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-8" 9'-7" 9'-1" TO 10'-1" 10'-1"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95	18 1 TYPE 21 41 STR STR STR 18 18	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-8"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-1" TO 3'-7" 3'-7" 2'-10"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-1" TO 3'-7" 3'-7" 2'-10"	DIM. D 1'-11"	DIM. E/RAD.		
FA701 FA801 FA801 P401 P402 P501 P502 P503 P504 P505 P506 P507 P508 P508 P601	18 18 18 18 18 18 18 9 6 21 2 6 21 2 5.0. 8 5 NOT USED 10 5	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-7" 9'-1" 10'-1" 10'-1" 10'-1" 10'-1"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 160 53 95 123	18 1 1 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-8" 0'-6"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-1" TO 3'-7" 3'-7" 2'-10" 5'-6"	TEEL L. DIM. C 11" 3'-4" 3'-7" 3'-7" 2'-10" 5'-3"	DIM. D	DIM. E/RAD.		
FA701 FA801 FA801 P401 P402 P501 P502 P503 P504 P505 P506 P505 P506 P507 P508 P508 P601 P602	18 18 18 18 18 9 6 2 9 6 21 2 5.0. 8 5 NOT USED 10 5 10	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-7" 9'-1" 10'-1" 10'-1" 10'-1" 10'-1" 10'-2" 9'-1" 16'-4" 6'-5"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 160 53 95 123 96	18 1 1 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-1" TO 3'-7" 3'-7" 2'-10"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P402 P501 P502 P503 P504 P505 P506 P507 P506 P507 P508 P507 P508 P601 P602 P901	18 18 18 18 18 18 9 6 2 9 6 21 2 5 .0. 8 5 NOT USED 10 5 10 40	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-8" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 16'-4" 16'-4" 16'-5" 14'-9"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006	18 1	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-1" TO 3'-7" 3'-7" 2'-10" 5'-6"	TEEL L. DIM. C 11" 3'-4" 3'-7" 3'-7" 2'-10" 5'-3"	DIM. D 1'-11"			
FA701 FA801 FA801 P402 P501 P502 P503 P504 P505 P506 P506 P507 P508 P507 P508 P507 P508 P507 P508 P5001 P602 P901 P1101	18 18 18 18 18 18 9 6 2 9 6 2 1 2 5 .0. 8 5 NOT USED 10 5 10 5 10 40 32	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-4" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-2" 10'-1" 10'-2" 10'-	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335	18 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 18 18 18 18 18 18 19	8'-8" 1'-4" EINF OR DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3"	TEEL L. DIM. C 11" 3'-4" 3'-7" 3'-7" 2'-10" 5'-3"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P401 P502 P503 P504 P505 P506 P506 P507 P508 P507 P508 P507 P508 P507 P508 P5001 P502 P501 P601 P602 P901 P1101 P1102	18 18 18 18 18 18 9 6 2 9 6 2 1 2 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 10 10 40 32 2	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-4" 9'-7" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335 177	18 1 1 1 1 1 1 1 1 1 1 1 1 18 18 18 18 18 18 19 STR	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3" 5'-4"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-7" TO 3'-7" 2'-10" 5'-3" 3'-6"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P402 P501 P502 P503 P504 P505 P506 P506 P507 P508 P507 P508 P507 P508 P507 P508 P5001 P602 P901 P1101	18 18 18 18 18 18 9 6 2 9 6 2 1 2 5 .0. 8 5 NOT USED 10 5 10 5 10 40 32	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 9'-7" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 10'-2" 25'-6" 14'-9" 25'-6" 16'-8" 20'-0"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335 177 850	18 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 18 18 18 18 18 18 19	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3"	TEEL L. DIM. C 11" 3'-4" 3'-7" 3'-7" 2'-10" 5'-3"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P401 P502 P503 P504 P505 P506 P506 P506 P507 P508 P507 P508 P507 P508 P500 P508 P500 P500 P500 P500 P500	18 18 18 18 18 18 9 6 2 9 6 2 1 2 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 10 10 40 32 2	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-4" 9'-7" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2" 10'-2"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335 177	18 1 1 1 1 1 1 1 1 1 1 1 1 18 18 18 18 18 18 19 STR	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3" 5'-4"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-7" TO 3'-7" 2'-10" 5'-3" 3'-6"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P402 P501 P502 P503 P504 P505 P506 P507 P506 P507 P506 P507 P508 P601 P602 P901 P1001 P1102 P1103	18 18 18 18 18 18 2 9 6 21 2 5 .0. 8 5 NOT USED 10 5 10 40 32 2 8 8	12'-0" 9'-0" TOTAL 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 16'-8" 9'-7" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 10'-6" 10'-1" 10'-6" 10'-6" 10'-6" 10'-6" 14'-9" 25'-6" 16'-8" 20'-0" TOTAL	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335 177 850 8398	18 1 1 7YPE 21 41 STR STR 18 18 18 18 18 26 20 40 19 STR 18	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 1'-4"	1'-10" 7'-11" DIM. B 8" 1'-8" 3'-1" TO 3'-7" 3'-7" 2'-10" 5'-6" 3" 5'-4"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-7" TO 3'-7" 2'-10" 5'-3" 3'-6"	DIM. D 1'-11"			STD.
FA701 FA801 FA801 P401 P502 P503 P504 P505 P506 P506 P507 P508 P507 P508 P507 P508 P507 P508 P507 P508 P501 P508 P601 P602 P901 P1101 P1102	18 18 18 18 18 18 9 6 2 9 6 2 1 2 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 8 5 .0. 10 10 40 32 2	12'-0" 9'-0" TOTAL LENGTH 1'-6" 5'-8" 12'-8" 16'-4" 16'-4" 9'-7" 9'-1" TO 10'-1" 10'-1" 10'-1" 10'-1" 10'-2" 25'-6" 14'-9" 25'-6" 16'-8" 20'-0"	442 433 3040 PIE WEIGHT 42 34 79 34 104 210 160 53 95 123 96 2006 4335 177 850	18 1 1 1 1 1 1 1 1 1 1 1 1 18 18 18 18 18 18 19 STR	8'-8" 1'-4" DIM. A 4" 1'-0" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2" 3'-2"	1'-10" 7'-11" CING S DIM. B 8" 1'-8" 3'-4" 3'-7" 3'-7" 3'-7" 2'-10" 5'-6" 3" 5'-4"	1'-10" TEEL L. DIM. C 11" 3'-4" 3'-7" TO 3'-7" 2'-10" 5'-3" 3'-6"	DIM. D 1'-11"			std.

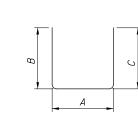
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DESCRIPTION

ISSUE RECORD: NO. DATE

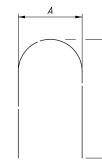
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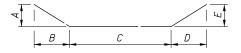
<u>TYPE 18</u>

<u>TYPE 20</u>

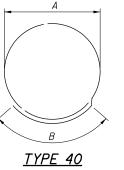


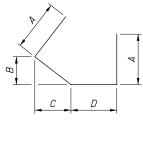
В

<u>TYPE 21</u>



<u>TYPE 26</u>





<u>TYPE 41</u>

<u>NOTES:</u> 1. ALL REINFO	10/06/2021		300
1. ALL REINFO		2	, o
BAR MARK. LETTERS O LOCATION. THE BAR SI	DRCING STEEL SHALL BE EPOXY COATED. THE BAR SIZE IS INDICATED IN THE THE MARK BEGINS WITH TWO OR THREE R NUMBERS THAT IDENTIFY THE BAR THE NEXT ONE OR TWO DIGITS INDICATE IZE, AND THE REMAINING TWO DIGITS ARE NCE NUMBER.	DESIGN AGENC	8415 Pulsar Place Suit Columbus Ohio 43240
10 = 01 = 1	= SUPERSTRUCTURE BAR #10 BAR BAR SEQUENCE NUMBER 1	DATE 6/11/21	FILE NUMBER 2950
UNLESS OT	SIONS SHOWN ARE OUT-TO-OUT HERWISE INDICATED.		STRUCTURE FILE NI 7702950
STRAIGHT E		REVIEWED	STRUC
5. RAD. INDIC OTHERWISE	ATES INSIDE RADIUS, UNLESS NOTED.	DRAWN JAT	REVISED
6. INCR. INDIC SERIES BAF	RS.		
	TEN IN PLACE OF A DIMENSION A STANDARD BEND AT THE END OF		CHECKED KDC
WEIGHTS . ONLY. CC REINFORC		REINFORCING STEEL L	BRIDGE NO. SUM-77-1181 RUBBER CITY HERITAGE TRAI BRIDGE OVER RAMP N, LANE M, S.R. 8, AND LANE O
	2021-09-16_BU 12 - RFC PLANS	10 0 12 16 12 8 16 12 8 16 16 16 16 16 16 16 16 16 16 16 16 16	91 PID No. 101402
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