

**REPORT  
OF  
SUBSURFACE INVESTIGATION  
FOR  
LICKING THORNWOOD DRIVE PROJECT  
LICKING COUNTY, OHIO  
(PID #78116)**

Prepared for:

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DLZ Job No. 0821-3004.00

October 12, 2009

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**REPORT  
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LICKING THORNWOOD DRIVE PROJECT  
LICKING COUNTY, OHIO**

**I. Introduction**

This report presents the findings of a subsurface investigation performed by DLZ Ohio, Inc. for the Licking County Thornwood Drive reconstruction project. This project is located in the City of Newark and begins approximately 500 feet north of the intersection of Thornwood Drive and Beaver Run Road. It ends near the intersection Reddington Road and Cherry Valley Road. The total length of the project is approximately 4 miles.

The scope of the project involves the widening and re-alignment of Thornwood Drive to provide standard lane widths, graded shoulders, upgraded horizontal and vertical alignments, and clear zone grading where possible. In addition, left turn lanes will be provided on Thornwood Drive at relocated River Road, West Main Street and James Road. The following intersecting roads will be widened to accommodate left turn lanes: West Main Street, James Road, and Lees Road. The profile grade along most of the Thornwood Drive is not anticipated to change by more than a few feet. However, fills of up to 12 feet may be required near the existing railroad tracks (between Stations 134+00.00 and 157+00.00), the areas between Stations 175+00.00 and 195+00.00 and in the existing ditch lines.

The geotechnical engineer has planned and supervised the performance of the geotechnical engineering services, considered the findings, and prepared this report in accordance with generally accepted geotechnical engineering practices. No other warranties, either expressed or implied, are made as to the professional advice included in this report.

**II. Field Exploration**

The original subsurface investigation consisted of drilling 71 borings, B-001 through B-071. However, due to a change in the original scope of work, B-021-1, B-028-1, and B-042-1, and B-060-1 were added and borings B-057, B-060, B-062, B-064 and B-065 were eliminated from the original plan. Borings B-002, B-021-1, B-028-1, B-041 and B-042, B-058, and B-060-1 were drilled in the proposed ditches and sidehill fill areas. As a result, a total of 63 roadway borings were drilled for the pavement subgrade evaluation. A majority of the roadway borings were drilled on Thornwood Drive. However, of the 63 roadway borings, two borings, B-030 and B-031, were drilled on Lees Road while four borings, B-051 through B-054, were drilled on James Road/Main Street and boring B-066 was drilled on Reddington Road Connector. In addition, five roadway borings, B-067 through B-071, were drilled on Reddington Road. The borings were generally drilled to depths between 6.5 and 22.5 feet deep, with the majority of them drilled to a depth of 10.0 feet. However, boring B-042 was drilled to a depth 42.0 feet.

The borings were drilled on April 2, 2009 and August 27, 2009, using an all-terrain vehicle mounted or truck mounted, rotary-type drill rig. Additional information regarding the drilling procedures are presented in Appendix I.

During the subsurface exploration, several borings encountered very soft and organic soils. In accordance with the Specifications for Geotechnical Explorations, the existing borings should have been extended and additional borings should have been drilled to refine the limits of the poor soils. In April 2009, DLZ informed HNTB that we intended to stop work on the project until we received direction about the extent of the additional exploration, since the additional drilling and analyses would be beyond the original scope of services.

On July 28, 2009, DLZ met with ODOT District 5 personnel to discuss the additional work based on the presence of the poor soils. The ODOT personnel indicated that additional funds were not available at this time for significant additional drilling and analyses regarding the poor soils. Therefore, it was decided that DLZ would drill a few additional borings and perform testing but that stability and settlement analyses would be performed at a later time by others. The results of the few additional borings and laboratory testing are presented in this report. The exceptions to this are the results of consolidation tests which will be completed later and submitted under separate cover.

DLZ prepared a letter dated August 3, 2009 that documented the results of the meeting on July 28, 2009. A copy of this letter is presented in Appendix I.

Boring locations were determined by DLZ Ohio, Inc. personnel and initially marked in the field by Jobes-Henderson & Associates personnel. Subsequent additional borings or borings that required location adjustment were marked in the field by DLZ Ohio, Inc. personnel. The approximate locations of the borings, in reference to the station numbers on the project plans, are shown on the Boring Logs in Appendix I. The ground surface elevations at all of the borings were estimated using the site plans provided by HNTB at Cleveland Office, between August and September of 2009. The estimated elevations at the boring locations are included on the Boring Logs.

### **III. Geology of the Site**

The project area is located within the glaciated portion of the Allegheny Plateau Physiographic Province. The site also lies east of the Allegheny Escarpment, which bisects the Licking County from north to south. This physiographic region is characterized by ridges and flat uplands, which are covered by a thin layer of glacial drift and dissected by steep valleys. The project area is located on an outwash terrace.

The primary drainage in the area is provided by the Licking River and associated tributaries. Thornwood Drive is located immediately west of the South Fork of the Licking River.

Subsurface information obtained from the Groundwater Resources Map of Licking County indicates that the project lies in a large buried valley aquifer system of the ancient Newark River. The outwash deposits that filled the valley are in excess of 200 feet in thickness. Information suggests that highly productive water zones are present in the valley fill. Yields of between 100 and 500 gallons per minute can be derived from these deposits.

#### **IV. Subsurface Conditions**

The following sections present the generalized subsurface conditions encountered by the borings. For more detailed information, refer to the Boring Logs presented in Appendix I. Results of laboratory tests are shown on the Boring Logs in Appendix I.

##### **A. Soil Conditions**

At the ground surface, the borings drilled on the existing pavement areas generally encountered between 5 and 15 inches of asphalt concrete while the borings drilled outside of the existing roadway encountered between 2 and 9 inches of topsoil or between 1 and 6 inches of aggregate. However, asphalt concrete, topsoil or aggregate were not encountered at the ground surface of borings B-002, B-010, B-019 and B021. Below the surface materials, the borings generally encountered cohesive materials mostly consisting of Sandy Silt (A-4a), Silt (A-4b), Silt and Clay (A-6a), Silty Clay (A-6b). Granular materials, mostly Gravel with Sand (A-1-b), were also encountered in the borings. Occasionally, Clay (A-7-6), Gravel (A-1-a), Coarse and Fine Sand (A-3a), Gravel with Sand and Silt (A-2-4) were also encountered in isolated borings. Organic Silt (A-8a) and Organic Clay (A-8b) was present at depths between 12 inches and 8.5 feet in borings B-001, B-005, B-011, B-014, B-018 and B-020. These organic silt soils contained organic contents ranging from 5.4 to 11.4 percent. Clay (A-7-6) and Silty Clay (A-6b) with high organic contents, (but not classified as organic soils) were encountered between depths of 1.5 and 7.0 feet in borings B-021, B-028, B-029 and B-033. The organic contents of these cohesive soils ranged from 4 to 8 percent. Bedrock was not encountered in the majority of borings within the depths of investigation. However, highly weathered and decomposed sandstone was encountered between depths of 2.0 feet (Elevation 965.6) and 8.0 feet (Elevation 1012.5) in borings B-055, B-056, B-058, B-60-1, B-061, and B-063. These borings are located near the north end of the project area.

Standard penetration blow counts (N-values) in the subgrade soils, within seven feet of the estimated finished grade of the proposed roadway, varied from WOH (weight of hammer) to 83 blows per 12 inches of penetration, with an average N-value of approximately 13, indicating very loose to dense or very soft to hard material. The higher blow counts may be due to the presence of gravel or rock fragments occasionally encountered in the soil. The natural moisture contents of the subgrade soils ranged from

5% to 89%, with an average of 9.5%. The high moisture contents generally were present in soils containing high organic contents.

**B. Groundwater Conditions**

Seepage was first encountered at depths between 3.0 and 18.5 feet, with elevations ranging from 964.9 to 880.2 feet. Groundwater levels were observed at depths between 5.0 and 18.8 feet, with elevations ranging from 949.0 to 870.5 feet. It should be noted that water levels may fluctuate due to rainfall, surface drainage, site topography, and other climatic factors. During construction or at other times during the project life, the water level may be higher or lower than observed at the time of the investigation.

**V. Conclusions and Recommendations**

**A. Pavement Design Information**

A summary of the subgrade soils encountered by the borings, the average group indices, CBR values, and number of samples for each soil type are listed in Table 1 below. It should be noted that only information pertaining to samples within the upper seven feet of the anticipated subgrade is presented.

*Table 1.*

<b>Subgrade Soils</b>			
<b>ODOT Classification</b>	<b>Group Index</b>	<b>CBR</b>	<b>No. Samples</b>
A-1-a	0	12	7
A-1-b	0	12	29
A-2-4	0	12	13
A-3	0	12	3
A-3a	0	12	2
A-4a	5	8	53
A-4b	8	6	22
A-6a	9	6	67
A-6b	11	5	37
A-7-6	15	4	9
A-8a	20	0	6
A-8b	20	0	2

As indicated, the group indices of the soils varied from 0 to 19, with an average of 6.5, which correlates with a CBR value of 7.

## **B. Subgrade Recommendations**

The existing subgrade soils at the boring locations were evaluated for suitability according to the ODOT Geotechnical Bulletin GB-1 "Plan Subgrade." It should be noted that only samples within 7 feet of the estimated finished grade were evaluated. The GB-1 Soil Investigation Summary Sheet is included in Appendix II.

According to the GB-1, any soil with moisture content exceeding the optimum moisture content of the soil by three or more percentage points, or has low N-values, is considered problematic soil and will likely require some form of subgrade treatment. The stabilization options available include undercutting, cement stabilization, and lime stabilization. However, the cement or lime stabilization may not be effective in very weak soils (N-values less than 5 blows per foot).

Results of this surface investigation indicate that approximately 48 percent of the samples, located within 7 feet of the estimated finished grade, had moisture contents exceeding their corresponding optimum moisture contents by more than 3 percent and that approximately 59 percent of the samples had low N-values. The natural moisture contents of the subgrade soil ranged from 5% to 89%, with an average of 19.2%, and the N-values varied from WOH to 83, with an average of 13. The low N-value of each boring varied from WOH to 20, with an average of 7.5. Silt (A-4b), Organic Silt (A-8a) and Organic Clay (A-8b) were present within the subgrade elevations in isolated boring locations. As a result, subgrade stabilization is required for portions of the project area.

In accordance with the results of the GB-1 analysis, chemical stabilization of 12 to 16 inches of subgrade soil at 24 boring locations in conjunction with undercutting of 1 to 5 feet at 15 boring locations is an appropriate subgrade treatment. Alternatively, undercutting of 1 to 5 feet in 39 of the 63 boring locations may be considered. Based on these results, subgrade stabilization is required of approximately 62 percent of the project areas.

Based on the soil information, organic soils were present within the subgrade elevations in isolated boring locations. It is probable that organic soils may be encountered at other locations along the proposed alignment that were not disclosed by the borings. Since chemical stabilization may not be effective in organic soils, undercutting of 1 to 5 feet of existing subgrade appears to be a preferred option. The undercut area should be replaced with compacted Type B or Type C granular material (ODOT Item 703.16.C) and should extend 18 inches beyond the edge of the pavement, paved shoulders, or paved medians. Before placing the compacted granular material, proofroll the entire undercut area. If any areas rut or deflect, overexcavate the soft or loose soils and replace with compacted Type B or Type C granular material. Please note that the ODOT Geotechnical Bulletin GB-1 requires that a geotextile fabric Type D (ODOT Item 712.09) be placed in the bottom of

the undercut for separation purposes. A summary of the recommended depths of undercuts at each of the treatment areas is presented in Table 2 below.

*Table 2.*

Treatment Areas		Referencing Boring Locations	Recommended Depth of Undercuts, ft.
Beginning Station*	Ending Station*		
6+00.00	10+00.00	B-001	7.0 <sup>1</sup>
14+00.00	18+00.00	B-003	1.0
22+00.00	29+50.00	B-005, B-006	3.0
31+50.00	39+50.00	B-008, B-009	3.0
46+50.00	50+50.00	B-012	3.0
54+50.00	58+50.00	B-014	5.0 <sup>1</sup>
58+50.00	66+50.00	B-015 through B-017	3.0
70+50.00	78+50.00	B-019 through B-021	5.0 <sup>2</sup>
82+50.00	92+50.00	B-022, B-023, B-024	3.0
101+50.00	109+50.00	B-028, B-029	3.0
118+50.00	130+50.00	B-035, B-036, B-037	3.5
134+50.00	138+50.00	B-039	3.0
162+50.00	166+50.00	B-046	3.0
166+50.00	170+50.00	B-047	5.0 <sup>2</sup>
174+50.00	178+50.00	B-049	2.0
194+50.00	198+50.00	B-061	2.0
13+00.00 (Lees Rd)	17+00.00 (Lees Rd)	B-030	3.0
17+00.00 (Lees Rd)	21+00.00 (Lees Rd)	B-031	5.0 <sup>2</sup>
13+00.00 (James/Main St)	17+00.00 (James/Main St)	B-051	1.0
19+00.00 (James/Main St)	23+00.00 (James/Main St)	B-053	5.0 <sup>2</sup>
21+00.00 (Reddington Rd Connector)	25+00.00 (Reddington Rd Connector)	B-066	5.0 <sup>2</sup>
18+00.00 (Reddington Rd)	22+00.00 (Reddington Rd)	B-067	5.0 <sup>2</sup>
22+00.00 (Reddington Rd)	25+00.00 (Reddington Rd)	B-068	2.0

\*Stationing is referenced to Thornwood Drive unless noted otherwise.

<sup>1</sup>Deeper undercut is recommended to remove the underlying organic soils.

<sup>2</sup>Undercutting of 5 feet due to the existence of organic soils or soils with N-values less than 5.



According to GB-1, for projects where it is determined that 30 percent or more of the subgrade area must be stabilized, consideration should be given to stabilizing the entire project. If the owner chooses to treat the subgrade of the entire project, it is recommended that an overall subgrade treatment of 3.0 feet of undercutting be performed, except where organic soils or soils with N-values less than 5 are present the undercutting extend deeper as noted in Table 2.

Given the high moisture contents in the existing subgrade soils and the low permeability of in-place subgrade, underdrain systems should be considered for the proposed pavement.

### **C. Sidehill Fill Design**

The ODOT Geotechnical Bulletin GB-2, *Special Benching and Sidehill Embankment Fills*, was used to determine the cut angles and the stability of the proposed sidehill fill between Stations 130+50 and 130+75 and also between Stations 190+50 and 197+50. Results of the special benching analyses are included in Appendix III. It should be noted that the evaluation of the embankment stability is not within the scope of this subsurface investigation.

## VII. Closing Remarks

We appreciate having the opportunity to be of service to you on this project. Please do not hesitate to call if you have any questions concerning our report.

Respectfully submitted,

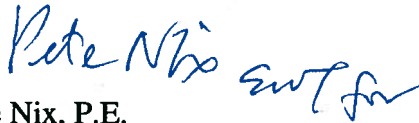
DLZ OHIO, INC.



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## **APPENDIX I**

August 3, 2009 Letter from DLZ to HNTB  
General Information – Drilling Procedures and Logs of Borings  
Legend – Boring Log Terminology  
Boring Logs – Seventy (70) Borings



August 3, 2009

HNTB  
1100 Superior Avenue, Ste. 1330  
Cleveland, OH 44114

Attn: Mr. Jon Lorincz, P.E.

Re: Subsurface Exploration Status  
LIC-Thornwood Drive  
DLZ Job No. 0821-3004.00

Mr. Lorincz:

This letter supplements our April 28, 2009 and June 19, 2009 letters that provided information about the geotechnical effort to date, the findings from the borings, and the costs we have incurred on the project. This letter is provided to clarify the geotechnical scope and budget for the remainder of the project. This clarification was the result of a meeting on July 28, 2009 between the undersigned and Nikunj Kadakia and Dave Slatzer from ODOT District 5.

Based on these discussions, DLZ will perform the following items of work to complete this phase of the subsurface exploration for the project. DLZ will perform this work within the existing, original budget for the project (\$95,463):

- DLZ will complete the remaining borings on the project at roughly 400-foot centers. Sampling will be performed in accordance with GB-1. In addition, DLZ will extend a roadway boring near the proposed culvert location to a depth of 20 feet. DLZ will also drill four additional borings to depths of 20 feet to develop two cross-sections in the area where soft, organic soils were encountered.
- A law enforcement office will be used for traffic control during the drilling operations; this effort wasn't included in the original cost estimate but we believe we can complete the work and still remain within the existing budget.
- DLZ will perform the subgrade evaluation in accordance with GB-1; this work was included in the original geotechnical scope of work.
- DLZ will evaluate the cut slopes; this work was also included in the original geotechnical scope of work.
- DLZ **will not** evaluate the embankments that will be founded on the soft, organic soils beyond what is required as part of the subgrade evaluation. It is anticipated that settlement and slope stability analyses will be required and that these analyses will be performed by others. Two consolidation tests will be performed as part of the existing exploration to be used later in these analyses.

August 3, 2009  
Subsurface Exploration Status  
LIC-Thornwood Drive  
Page -2-

We hope this information is helpful and clarifies the remaining geotechnical scope of work for the project. If you have any questions or need any additional information, please don't hesitate to contact us.

Sincerely,

DLZ OHIO, INC.

A handwritten signature in cursive script, appearing to read "Pete Nix".

Pete Nix, P.E.  
Geotechnical Engineering and Drilling Division Manager

## **GENERAL INFORMATION DRILLING PROCEDURES AND LOGS OF BORINGS**

Drilling and sampling were conducted in accordance with procedures generally recognized and accepted as standardized methods of investigation of subsurface conditions concerning geotechnical engineering considerations. Borings were drilled with either a truck-mounted or ATV-mounted drill rig.

Drive split-barrel sampling was performed in 1.5 foot increments at intervals not exceeding 5 feet. In the event the sampler encountered resistance to penetration of 6 inches or less after 50 blows of the drop hammer, the sampling increment was discontinued. Standard penetration data were recorded and one or more representative samples were preserved from each sampling increment.

In borings where rock was cored, NXM or NQ size diamond coring tools were used.

In the laboratory all samples were visually classified by a geotechnical engineer. Moisture contents of representative fine-grained soil samples were determined. A limited number of samples, considered representative of foundation materials present, were selected for performance of grain-size analyses and plasticity characteristics tests. The results of these tests are shown on the boring logs.

The boring logs included in the Appendix have been prepared on the basis of the field record of drilling and sampling, and the results of the laboratory examination and testing of samples. Stratification lines on the boring logs indicating changes in soil stratigraphy represent depths of changes approximated by the driller, by sampling effort and recovery, and by laboratory test results. Actual depths to changes may differ somewhat from the estimated depths, or transitions may occur gradually and not be sharply defined. The boring logs presented in this report therefore contain both factual and interpretative information and are not an exact copy of the field log.

Although it is considered that the borings have disclosed information generally representative of site conditions, it should be expected that between borings conditions may occur which are not precisely represented by any one of the borings. Soil deposition processes and natural geologic forces are such that soil and rock types and conditions may change in short vertical intervals and horizontal distances.

Soil/rock samples will be stored at our laboratory for a period of six months. After this period of time, they will be discarded, unless notified to the contrary by the client.

## LEGEND – BORING LOG TERMINOLOGY

Explanation of each column, progressing from left to right

1. Depth (in feet) – refers to distance below the ground surface.
2. Elevation (in feet) – is referenced to mean sea level, unless otherwise noted.
3. Standard Penetration (N) – the number of blows required to drive a 2-inch O.D., 1-3/8 inch I.D., split-barrel sampler, using a 140-pound hammer with a 30-inch free fall. The blows are recorded in 6-inch drive increments. Standard penetration resistance is determined from the total number of blows required for one foot of penetration by summing the second and third 6-inch increments of an 18-inch drive.  
  
50/n – indicates number of blows (50) to drive a split-barrel sampler a certain number of inches (n) other than the normal 6-inch increment.
4. The length of the sampler drive is indicated graphically by horizontal lines across the “Standard Penetration” and “Recovery” columns.
5. Sample recovery from each drive is indicated numerically in the column headed “Recovery”.
6. The drive sample location is designated by the heavy vertical bar in the “Sample No., Drive” column.
7. The length of hydraulically pressed “Undisturbed” samples is indicated graphically by horizontal lines across the “Press” column.
8. Sample numbers are designated consecutively, increasing in depth.
9. Soil Description
  - a. The following terms are used to describe the relative compactness and consistency of soils:

**Granular Soils – Compactness**

<u>Term</u>	<u>Blows/Foot Standard Penetration</u>
Very Loose	0 – 4
Loose	4 – 10
Medium Dense	10 – 30
Dense	30 – 50
Very Dense	over 50

**Cohesive Soils – Consistency**

<u>Term</u>	<u>Unconfined Compression tons/sq.ft.</u>	<u>Blows/Foot Standard Penetration</u>	<u>Hand Manipulation</u>
Very Soft	less than 0.25	below 2	Easily penetrated by fist
Soft	0.25 – 0.50	2 – 4	Easily penetrated by thumb
Medium Stiff	0.50 – 1.0	4 – 8	Penetrated by thumb with moderate pressure
Stiff	1.0 – 2.0	8 – 15	Readily indented by thumb but not penetrated
Very Stiff	2.0 – 4.0	15 – 30	Readily indented by thumb nail
Hard	over 4.0	over 30	Indented with difficulty by thumb nail

- b. Color – If a soil is a uniform color throughout, the term is single, modified by such adjective as light and dark. If the predominant color is shaded by a secondary color, the secondary color precedes the primary color. If two major and distinct colors are swirled throughout the soil, the colors are modified by the term “mottled”.
- c. Texture is based on the Ohio Department of Transportation Classification System. Soil particle size definitions are as follows:

<u>Description</u>	<u>Size</u>	<u>Description</u>	<u>Size</u>
Boulders	Larger than 8"	Sand – Coarse	2.0 mm to 0.42 mm
Cobbles	8" to 3"	– Fine	0.42 mm to 0.074 mm
Gravel – Coarse	3" to ¾"	Silt	0.074 mm to 0.005 mm
– Fine	¾" to 2.0 mm	Clay	smaller than 0.005 mm

- d. The main soil component is listed first. The minor components are listed in order of decreasing percentage of particle size.
- e. Modifiers to main soil descriptions are indicated as a percentage by weight of particle sizes.
 

trace	0 to 10%
little	10 to 20%
some	20 to 35%
"and"	35 to 50%

f. Moisture content of **cohesionless soils** (sands and gravels) is described as follows:

<u>Term</u>	<u>Relative Moisture or Appearance</u>
Dry	No moisture present
Damp	Internal moisture, but none to little surface moisture
Moist	Free water on surface
Wet	Voids filled with free water

g. The moisture content of **cohesive soils** (silts and clays) is expressed relative to plastic properties.

<u>Term</u>	<u>Relative Moisture or Appearance</u>
Dry	Powdery
Damp	Moisture content slightly below plastic limit
Moist	Moisture content above plastic limit but below liquid limit
Wet	Moisture content above liquid limit

10. Rock Hardness and Rock Quality Designation

a. The following terms are used to describe the relative strength of the **bedrock**.

<u>Term</u>	<u>Description</u>
Very Weak	Core can be carved with a knife and scratched by fingernail. Can be excavated readily with a point of a pick. Pieces 1-inch or more in thickness can be broken by finger pressure.
Weak	Core can be grooved or gouged readily by a knife or pick. Can be excavated in small fragments by moderate blows of a pick point. Small, thin pieces can be broken by finger pressure.
Slightly Strong	Core can be grooved or gouged 0.05 inch deep by firm pressure of a knife or pick point. Can be excavated in small chips to pieces about 1-inch maximum size by hard blows of the point of a geologist's pick.
Moderately Strong	Core can be scratched with a knife or pick. Grooves or gouges to 1/4" deep can be excavated by hand blows of a geologist's pick. Requires moderate hammer blows to detach hand specimen.
Strong	Core can be scratched with a knife or pick only with difficulty. Requires hard hammer blows to detach hand specimen. Sharp and resistant edges are present on hand specimen.
Very Strong	Core cannot be scratched by a knife or sharp pick. Breaking of hand specimens requires hard repeated blows of the geologist hammer.
Extremely Strong	Core cannot be scratched by a knife or sharp pick. Chipping of hand specimens requires hard repeated blows of the geologist hammer.

b. Rock Quality Designation, RQD – This value is expressed in percent and is an indirect measure of rock soundness. It is obtained by summing the total length of all core pieces which are at least four inches long, and then dividing this sum by the total length of the core run.

- 11. Gradation – when tests are performed, the percentage of each particle size is listed in the appropriate column (defined in Item 9c).
- 12. When a test is performed to determine the natural moisture content, liquid limit moisture content, or plastic limit moisture content, the moisture content is indicated graphically.
- 13. The corrected standard penetration (N<sub>60</sub>) value in blows per foot is indicated graphically.







Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00														
LOG OF: Boring B-003			Location: Sta 15+96.33, 0.19 left			Date Drilled: 4/6/2009														
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro-meter (tsf)	WATER OBSERVATIONS: Water seepage at: 7.0-15.0 Water level at completion: 15.6 FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ / Non-Plastic - NP 10 20 30 40						
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay					
DESCRIPTION																				
1.0	903.1						Asphalt Concrete Pavement - 12"													
		6 8 5	15	1		2.5	Very stiff brown SILT AND CLAY (A-6a), trace fine sand; moist.		0	0	---	1	62	37						
		4 4	17	2		2.25														
4.0	900.1						Stiff brown SILT AND CLAY (A-6a), little fine to coarse sand, trace gravel; moist. @ 5.5', contains little gravel.		1	3	---	10	61	25						
		3 4 5	18	3		1.5														
		4 4		4			Very loose to loose brown GRAVEL WITH SAND (A-1-b), some to "and" fine to coarse sand; little silt; trace to little clay; wet.		38	25	---	12	15	10						
7.0	897.1	3 2 4	8	5																
		3 2 1	10	6																
		2 2 4	6	7																
		WOH WOH		8			Very stiff brownish-gray SILT AND CLAY (A-6a), little to some gravel, little fine to coarse sand; damp.													
15.0	889.1	2	3																	
		3 6 9	12	9		3.0														
		3 5		10		2.75														
20.0	884.1	7	14																	
Bottom of Boring - 20.0'																				

















Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00													
LOG OF: Boring B-011			Location: Sta 44+21.43, 1.78 right			Date Drilled: 4/9/2009													
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: None Observed Water level at completion: None FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ / Non-Plastic - NP 10 20 30 40					
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay				
	917.8																		
1.0	916.8						Asphalt Concrete Pavement - 12"												
2.5	915.3	2 3 7	10	1		2.5	POSSIBLE FILL: Very stiff brown SANDY SILT (A-4a), some fine to coarse sand, some gravel; little clay; damp.		24	12	---	14	35	15					
4.0	913.8	5 4 4	8	2		1.0	POSSIBLE FILL: Medium stiff to stiff brown SILTY CLAY (A-6b), little to some fine to coarse sand, little gravel; ; moist.		11	8	---	12	42	27					
4.5	913.3	2 3 4	16	3		1.5	Stiff black ORGANIC SILT (A-8a), little fine to coarse sand; "and" clay; moist. LOI @750 C = 12.8%		0	2	---	9	50	39					
		2 3 3	9	4		2.0	Stiff olive - brown SILTY CLAY (A-6b), trace fine sand; moist.												
		2 5 5	18	5		2.25	@ 5.5', mottled brown and light gray. @ 7.0', trace to little fine to coarse sand, trace gravel.												
		4 6 5	15	6		2.0	@ 8.5', color grades to brown.												
10																			
		3 4 6	18	7		2.0													
13.0	904.8																		
		5 6 10	18	8		4.5+	Hard brownish gray SANDY SILT (A-4a), little to some fine to coarse sand, trace to little gravel; some clay; damp.												
		5 8 10	18	9		4.5+			6	9	---	15	42	28					
		5 8 9	18	10		4.5+													
20.0	897.8																		
							Bottom of Boring - 20.0'												

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Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00													
LOG OF: Boring B-014			Location: Sta 56+46.16, 14.15 left			Date Drilled: 4/9/2009													
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetrometer (tsf)	WATER OBSERVATIONS: Water seepage at: None Observed Water level at completion: None FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL  -----  LL Blows per foot - ○ / Non-Plastic - NP					
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay				
913.3																			
1.0	912.3						Asphalt Concrete Pavement - 11.5"												
		14 16 12	12	1			POSSIBLE FILL: Medium dense to dense brown GRAVEL WITH SAND (A-1-b), some silty clay, some fine to coarse sand; damp.		48	18	--	12	--22--						
3.0	910.3	4		2A		0.5													
4.0	909.3	2 3	18	2B		0.5	Soft to medium stiff grayish brown SILT (A-4b), some clay, little fine to coarse sand, trace gravel; moist.		2	4	--	13	58 23						
		1 2	18	3		0.5	Soft to medium stiff black ORGANIC SILT (A-8a), trace to little fine to coarse sand; trace gravel; contains fibrous organic material (wood fragments); damp to moist. @ 4.0', LOI; 5.7% @ 440 C, 7.1% @ 750 C.		0	1	--	4	54 41						
		WOH WOH WOH	18	4	ST-1	0.25	@ 5.0'-7.0'; Shelby Tube #1; Starting Pressure 0 psi - End Pressure 200 psi; 15.5" recovery.		5	4	--	6	48 37						
8.0	905.3	WOH WOH WOH	18	5A		0.25	@ 5.0', LOI; 9.7% @ 440 C, 11.4% @ 750 C.												
		WOH WOH WOH	18	5B		0.25	Soft brown and gray SILT AND CLAY (A-6a), trace fine sand; wet.												
		WOH WOH WOH	18	6		0.25	@ 8.5', light gray.												
		WOH WOH WOH	18	7		0.25	@ 10.5', gray.												
							@ 13.0', trace organic silt.												
		1 1	18	8		0.25			0	0	--	1	77 22						
							@ 15.5', stiff to very stiff, trace to little organic silt, moist.												
		1 3 5	18	9		1.25													
		2 4	16	10		2.5													
20.0	893.3	7					Bottom of Boring - 20.0'												

68.1 2648  
52  
92.0 2003  
42

















Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00													
LOG OF: Boring B-018			Location: Sta 68+70.76, 5.33 left			Date Drilled: 4/8/2009													
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro-meter (tsf)	WATER OBSERVATIONS: Water seepage at: None Observed Water level at completion: None FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ / Non-Plastic - NP 10 20 30 40					
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay				
	909.5																		
1.0	908.5						Asphalt Concrete Pavement - 11.5"												
2.5	907.0	5 7 12	12	1		4.5	POSSIBLE FILL: Very stiff to hard brown SILT AND CLAY (A-6a), trace gravel, little to some fine to coarse sand; contains brick fragments; damp.		8	7	---	13	45	27					
4.0	905.5	8 4 6	6	2			POSSIBLE FILL: Loose to medium dense brown GRAVEL WITH SAND AND SILT (A-2-4), little fine to coarse sand, trace clay; damp.												
5.5	904.0	4 3 3	6	3		1.0	Soft to medium stiff brown SANDY SILT (A-4a), some fine to coarse sand, little gravel; moist.		16	11	---	13	40	20					
		2 2 2	10	4		0.5	Soft to medium stiff brown SANDY SILT (A-4a), some fine to coarse sand, little gravel; moist.		7	6	---	18	39	30					
		1 2 2	6	5	ST-1	0.5	Soft to medium stiff black ORGANIC CLAY (A-8b), little fine to coarse sand, little gravel; moist.		15	6	---	12	33	34					
8.5	901.0	2 1 3	18	6		0.5	LOI @750 c = 6.7% @ 7.5', encountered small cobble/large gravel particle.												
10.5	899.0						Soft to medium stiff brown and gray SILTY CLAY (A-6b), little fine to coarse sand, trace gravel; contains trace organic material (roots); damp to moist.												
		2 6 7	18	7		2.5	Very stiff to hard brown SILT AND CLAY (A-6a), trace to little fine to coarse sand, trace gravel; damp.												
		3 7 10	18	8		4.5+													
	15						@ 15.0', brownish gray.												
		8 10 11	16	9		4.5+													
		5 8 13	18	10		4.5+													
20.0	889.5						Bottom of Boring - 20.0'												



Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00													
LOG OF: Boring B-020			Location: Sta 76+46.32, 9.72 right			Date Drilled: 4/8/2009													
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro-meter (tsf)	WATER OBSERVATIONS: Water seepage at: 9.0, 9.5-9.6 Water level at completion: 8.2 FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL  -----  LL Blows per foot - ○ / Non-Plastic - NP					
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay				
DESCRIPTION																			
1.1	914.0						Asphalt Concrete Pavement - 13.5"												
3.0	912.1	20	14	1		1.25	Dense brown GRAVEL WITH SAND (A-1-b), "and" fine to coarse sand, some silty clay; damp.		38	22	---	15	--25--						
4.5	910.6	13	4	2		1.25	Stiff black ORGANIC CLAY (A-8b), trace fine to coarse sand, trace gravel; moist. LOI @ 750 C = 9.8%		2	1	---	8	53	36					
		3	5	3		1.5	Stiff mottled brown and gray SILTY CLAY (A-6b), trace fine sand; moist.												
		3	4	4		1.5			1	1	---	5	56	37					
		2	4	5		1.0	@ 7.5', trace gravel.												
		2	3	6		1.5	@ 9.0', brown, trace to little fine to coarse sand; damp to moist.												
10.5	904.6	5	18	6		1.5													
							Bottom of Boring - 10.5'												
15																			
20																			
25																			

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Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00									
LOG OF: Boring B-021			Location: Sta 80+46.53, 2.37 right			Date Drilled: 4/13/2009									
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro- meter (tsf)	WATER OBSERVATIONS: Water seepage at: None Observed Water level at completion: None  FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ●  PL ——— LL Blows per foot - ○ / Non-Plastic - NP	
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
1.5	917.7	2		1			Loose brown GRAVEL WITH SAND AND SILT (A-2-4), "and" fine to coarse sand, little silt; trace clay; damp.		38	20	---	16	19	7	
	916.2	3	12	2		1.0	Stiff dark brown CLAY (A-7-6), trace to little fine to coarse sand, trace gravel; contains organic material; moist. LOI @750 C = 5.6%; @ 3.0', black.		3	4	---	8	48	37	
		3	14	3		1.25			0	1	---	4	47	48	
5.5	912.2	5	12	4		1.25			6	1	---	4	46	43	
		3	13	5		0.75	Medium stiff mottled brown and gray SILTY CLAY (A-6b), trace to little fine to coarse sand, trace gravel; slightly organic; moist.								
8.0	909.7	2	10	6		3.0	Very stiff brown SILT AND CLAY (A-6a), trace to little fine to coarse sand, trace gravel; damp.								
		4	18	7		2.75									
12.5	905.2	5	18												
		7					Bottom of Boring - 12.5'								
15															
20															
25															





Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00															
LOG OF: Boring B-023				Location: Sta 88+46.56, 0.14 left				Date Drilled: 4/8/2009													
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro- meter (tsf)	WATER OBSERVATIONS: Water seepage at: 6.0'-7.5', 9.5'-9.6', 11.5'-12.0' Water level at completion: 9.8'	FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL <span style="margin-left: 20px;"> ----- </span> LL Blows per foot - ○ / Non-Plastic - NP						
				Drive	Press / Core					% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay					
DESCRIPTION																					
0.1	920.9	6		1																	
1.5	919.5	8	6	1																	
3.0	918.0	3	18	2		0.25															
		3	18	3		1.0															
6.0	915.0	3	18	4		1.5															
		3	18	5		1.0															
9.6	911.4	4	16	6		0.5															
		4	16	6		4.5															
		3		7A		4.5															
		4		7B		4.5															
		5	18	7C		4.5															
15.0	906.0	3	15	8		4.5															
		6	15	8		4.5															
Bottom of Boring - 15.0'																					









































Client: HNTB, ODOT District-5			Project: LIC-Thornwood Drive			Job No. 0821-3004.00												
LOG OF: Boring B-040			Location: Sta 140+46.37, 10.82 left			Date Drilled: 4/7/2009												
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro-meter (tsf)	WATER OBSERVATIONS: Water seepage at: 8.5-20.0 Water level at completion: 8.0 FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL  -----  LL Blows per foot - ○ / Non-Plastic - NP				
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay			
0.9	892.8						Asphalt Concrete Pavement - 11"											
2.5	891.2	12 8 5	13	1			Medium dense brown SANDY SILT (A-4a), some fine to coarse sand, some gravel, little clay; damp.		21	13	---	18	32	16				
		7 7 6	12	2			Stiff mottled brown and gray SILTY CLAY (A-6b), little fine to coarse sand, trace gravel; moist.											
		6 3 5	15	3		1.5			1	5	---	7	39	48				
		3 3 6	18	4		2.0												
		4 2 3	12	5		0.5	@ 7.0', soft to medium stiff, trace coarse sand, trace gravel.											
8.5	885.2	5 6 6	5	6			Loose to medium dense brown GRAVEL WITH SAND (A-1-b), some fine to coarse sand, little silty clay; wet.											
		7 4 4	9	7					54	15	---	12	--19--		NP			
		5 4 5	0	8														
		3 4 3	6	9														
		4 5 7	8	10														
20.0	873.7						Bottom of Boring - 20.0'											



Client: HNTB, ODOT District-5				Project: LIC-Thornwood Drive				Job No. 0821-3004.00												
LOG OF: Boring B-042				Location: Sta 148+47.31, 31.55 left				Date Drilled: 4/1/2009												
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.		Hand Penetro- meter (tsf)	WATER OBSERVATIONS: Water seepage at: 6.5 Water level at completion: 7.7  FIELD NOTES: Stationing referenced to Thornwood Dr alignment	Graphic Log	GRADATION					STANDARD PENETRATION (N60) Natural Moisture Content, % - ●  PL ——— LL Blows per foot - ○ / Non-Plastic - NP						
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay					
1.0	892.4						Asphalt Concrete Pavement - 7" Aggregate Base - 5"													
2.5	890.9	8	15	1		2.0	POSSIBLE FILL: Medium dense brown GRAVEL WITH SAND AND SILT (A-2-4), some silty clay, some fine to coarse sand; damp.		37	18	---	15	---	30						
		3	13	2		0.5		POSSIBLE FILL: Stiff to very stiff dark brown CLAY (A-7-6), some silt, little fine to coarse sand; damp. @ 4.0', brown, trace fine sand, moist. @ 5.5', soft to medium stiff.		0	4	---	14	32	50					
7.0	886.4	4	16	3			Loose brown SANDY SILT (A-4a), "and" gravel, some fine to coarse sand; trace clay; moist to wet.  @ 10.0', wet.													
		3	14	4																
		2	9	5			Very loose to loose brownish gray COARSE AND FINE SAND (A-3a), trace silty clay; moist to wet.  @ 15.5', dark gray, little silty clay, contains slight organic odor.													
		2	7	6																
13.0	880.4	1	6	7			Very loose brown and gray SILT (A-4b), little fine sand, trace clay; moist to wet. @ 20.5' - 28.0', contains black, organic lenses.		43	7	---	14	30	6						
		1	6	8																
		7	9	9																
18.0	875.4	2	14	10																
		1	14	11																
		WOH	18	12																
		WOH	18																	
		WOH	18																	
25	868.4	WOH	18																	

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**APPENDIX II**

Summary of Subgrade Conditions  
GB-1 Spreadsheet



**Subgrade Analysis**  
V. 9.09 08/10/07

Design **7**  
CBR **No**  
Global CS **Option**  
Global LS **No**

Classification Counts by Sample																	
R	1a	1b	3	3a	2-4	2-5	2-6	2-7	4a	4b	5	6a	6b	7-5	7-6	8a	8b
0	7	29	3	2	13	0	0	0	53	22	0	67	37	0	9	6	2
	3%	12%	1%	1%	5%				21%	9%		27%	15%		4%	2%	1%
0.0%	21.6%						78.4%										

Class @ Surface	
2-5	0
4b	8 13%
5	0
7-5	0
7-6	1 2%
8a	1 2%
8b	0
R	0

% Borings	
N <sub>L</sub> <= 5	30%
N <sub>L</sub> <= 10	86%
N <sub>L</sub> >= 20	2%
M+	89%
R	0%

% Surface	
43%	
14%	41%
% Borings	
90%	
6%	67%
25%	90%

Rig	ER
A	61
B	86
C	62
D	
E	
F	
G	
H	

63 Total Borings		Average	N <sub>60</sub>	N <sub>L</sub>	PI	Clay	M	M <sub>OPT</sub>	GI
		Maximum	13.0	7.5	15.3	26.5	19.2	12.2	6.48
		Minimum	83	20	152	49	103	77	48
			0	0	21	13	1	5	4
							5	5	6
									0

Licking County, Thornwood Drive, PID 78116													Standard Penetration		Physical Characteristics					Moisture		Classification		Comments		Problem		Treatments				Analysis
#	B #	Boring Location	Depth	To	Cut Fill	n <sub>2</sub>	n <sub>3</sub>	N <sub>m</sub>	Rig	N <sub>60</sub>	N <sub>L</sub>	LL	PL	PI	% Silt	% Clay	P 200	M	M <sub>OPT</sub>	Class	GI			w/ Class	w/ MN	LS	CS	UC Class	UC MN			

1	B-001	Sta 7+95.86 Thornwood Dr.	5.92 LT	1.0	2.5	8	4	12	A	12		26	16	10	29	19	48	13	11	4a	3	Organic 4.0'-7.0'			4b	MN												
				2.5	4.0	3	4	7		7								28	10	4b																		
				4.0	5.5	3	4	7		7		57	26	31	44	38	82	56	14	8a	19																	
				5.5	7.0	3	4	7		7	7							55		8a																		
2	B-003	Sta 15+96.33 Thornwood Dr.	0.19 LT	1.0	2.5	8	5	13	A	13		33	19	14	62	37	99	22	14	6a	10																	
				2.5	4.0	4	6	10		10								25	14	6a	8																	
				4.0	5.5	4	5	9		9		29	18	11	61	25	86	22	14	6a	8																	
				5.5	7.0	4	4	8		8	8							19	14	6a																		
3	B-004	Sta 20+01.83 Thornwood Dr.	4.82 LT	1.0	2.5	16	19	35	A	36		NP		NP	23		23	7	6	1b	0																	
				2.5	4.0	6	7	13		13								15	14	6a	8																	
				4.0	5.5	3	4	7		7		50	26	24	62	28	90	34	23	7-6	16																	
				5.5	7.0	4	6	10		10	7							28	14	6a																		
4	B-005	Sta 23+96.74 Thornwood Dr.	9.18 RT	1.0	2.5	5	7	12	A	12		50	25	25	47	29	76	24		8a	16	Organic 1.0'-2.5'																
				2.5	4.0	4	6	10		10								20	10	4b																		
				4.0	5.5	4	3	7		7		26	21	5	77	17	94	25	16	4b	8																	
				5.5	7.0	4	6	10		10	7							21	10	4b																		
5	B-006	Sta 27+46.31 Thornwood Dr.	8.85 LT	0.3	1.5	2	1	3	B	4		31	18	13	41	21	62	21	14	6a	7																	
				1.5	3.0	7	7	14		20								15	14	6a	8																	
				3.0	4.5	5	6	11		16		29	17	12	39	35	74	19	14	6a	9																	
				4.5	6.0	5	5	10		14	4							16	14	6a	8																	
6	B-007	Sta 29+39.49 Thornwood Dr.	0.09 RT	1.0	2.5	9	8	17	A	17		24	19	5	44	18	62	15	14	4a	5																	
				2.5	4.0	5	6	11		11								20	10	4a	5																	
				4.0	5.5	5	5	10		10		27	19	8	42	18	60	14	14	4a	5																	
				5.5	7.0	4	5	9		9	9							24	10	4a																		
7	B-008	Sta 33+46.31 Thornwood Dr.	5.96 LT	0.3	1.5	4	5	9	A	9		NP		NP	22	7	29	11	10	2-4	0																	
				1.5	3.0	4	4	8		8		22	19	3	64	15	79	20	14	4b	8																	
				3.0	4.5	3	3	6		6		33	19	14	58	37	95	26	14	6a	10																	
				4.5	6.0	5	7	12		12	6							23	14	6a	8																	
8	B-009	Sta 37+46.31 Thornwood Dr.	2.35 RT	1.2	3.0	4	4	8	A	8		27	19	8	68	29	97	23	14	4b	8																	
				3.0	4.5	3	6	9		9		35	19	16	55	40	95	22	16	6b	10																	
				4.5	6.0	5	7	12		12								17	10	4b																		
				6.0	7.5	5	5	10		10	8							20	10	4b																		
9	B-010	Sta 41+46.35 Thornwood Dr.	11.14 LT	0.0	1.5	3	4	7	B	10		40	24	16	56	35	91	36	19	6b	10																	
				1.5	3.0	5	6	11		16								19	14	6a	8																	
				3.0	4.5	5	6	11		16		29	18	11	63	33	96	20	14	6a	8																	
				4.5	6.0	5	5	10		14	10							18	10	4a	5																	
10	B-011	Sta 44+21.43 Thornwood Dr.	1.78 RT	1.0	2.5	3	7	10	A	10		26	17	9	35	15	50	14	12	4a	3	Organic 4.0'-4.5'																
				2.5	4.0	4	4	8		8		34	13	21	42	27	69	24	16	6b	11																	
				4.0	4.5	3	4	7		7		68	27	41	50	39	89	30		8a	20																	
				4.5	7.0	3	3	6		6	6							16		6b	10																	
11	B-012	Sta 48+46.32 Thornwood Dr.	11.54 LT	0.4	1.5	2	3	5	B	7		31	19	12	53	36	89																					









**APPENDIX III**

Sidehill Fill Design (GB-2)

M:\proj\0821\3004.00\Drawings\Final\78116XS001.dgn 10/12/2009 6:47:12 PM dadams

SEEDING

END WIDTH SQ. YDS.

END AREA

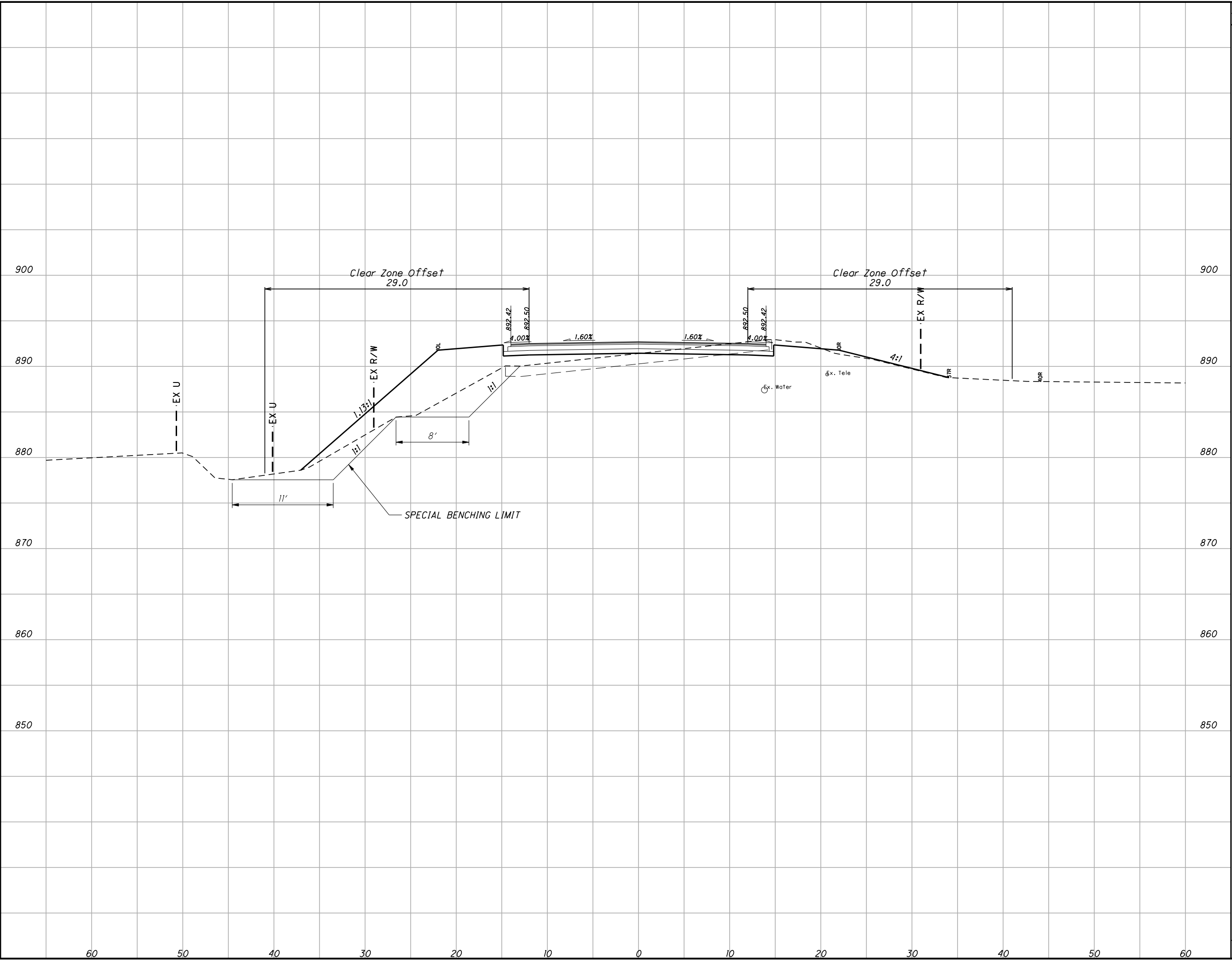
CUT FILL

VOLUME

CUT FILL

CALCULATED

CHECKED



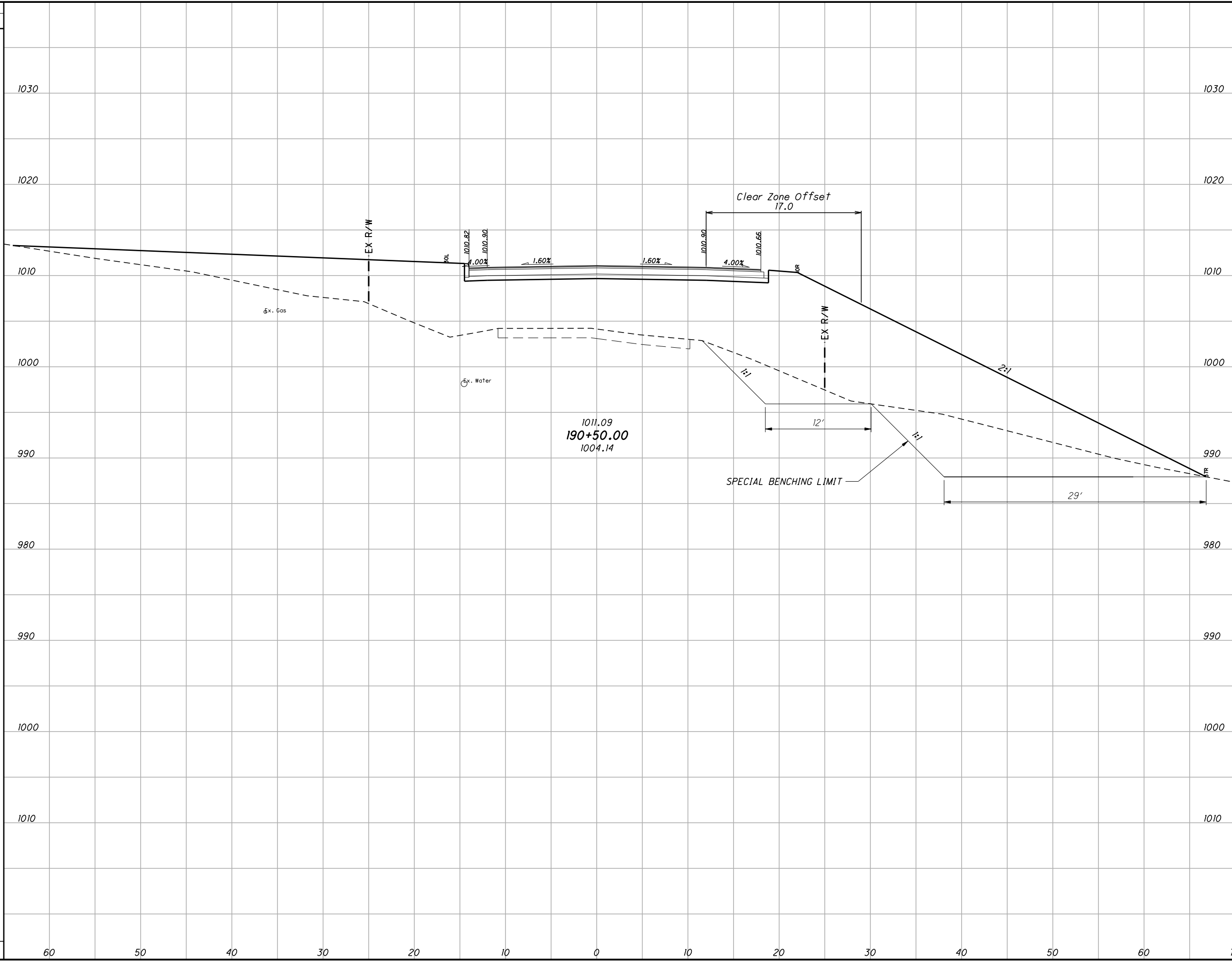
CROSS SECTION  
STATION 130+50

LIC-THORWOOD

M:\proj\0821\3004.00\Drawings\Final\78116XS002.dgn 10/13/2009 8:31:58 AM dodams

**SEEDING**

END WIDTH	SO. YDS.
60	
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	
70	



END AREA		VOLUME	
CUT	FILL	CUT	FILL

CALCULATED	CHECKED

**CROSS SECTION  
STATION 190+50**

**LIC-THORWOOD**



M:\proj\0821\3004.00\Drawings\Final\78116XS003.dgn 10/13/2009 8:35:03 AM dodoms

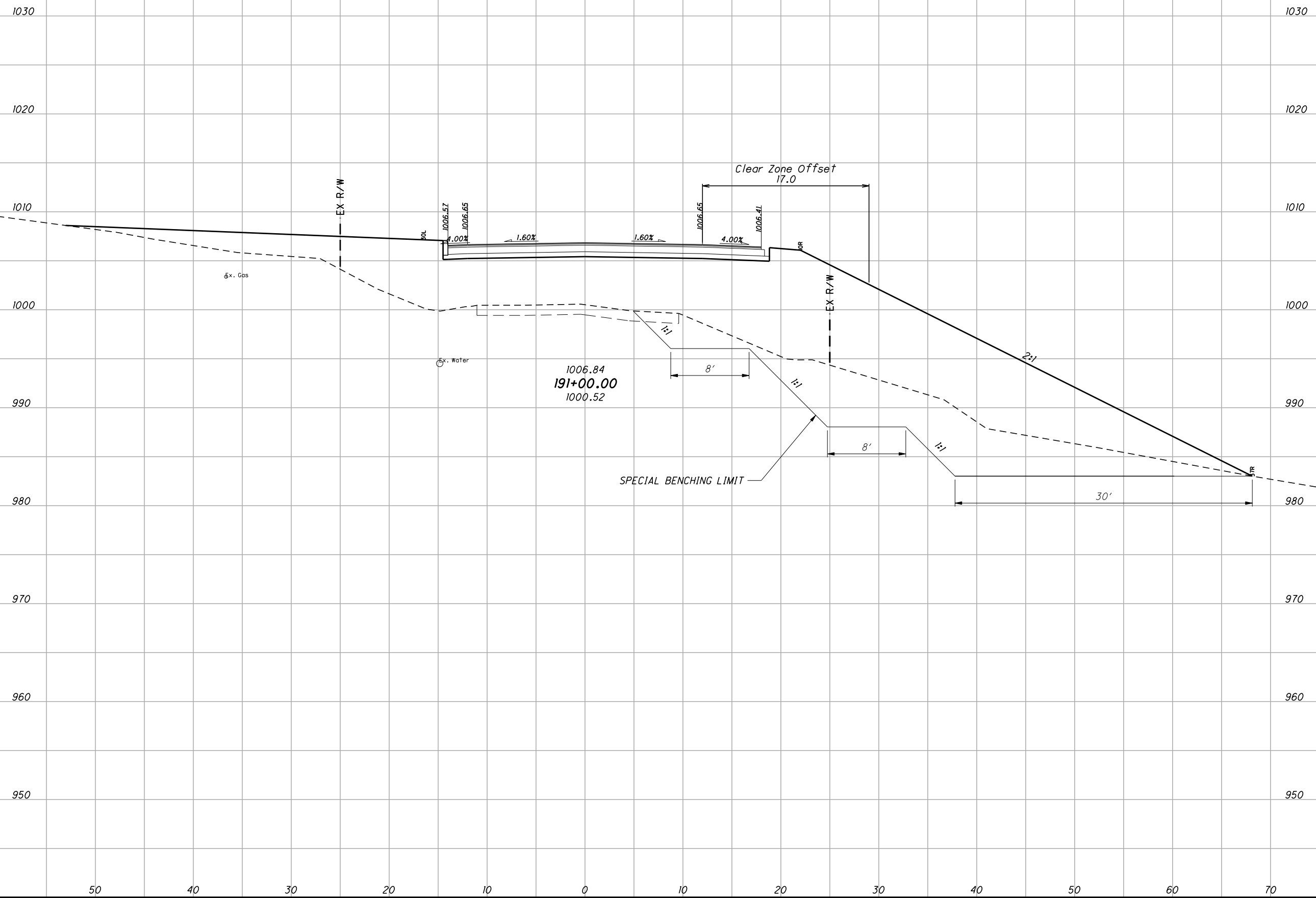
SEEDING

END WIDTH	SO. YDS.

END AREA VOLUME

CUT	FILL	CUT	FILL

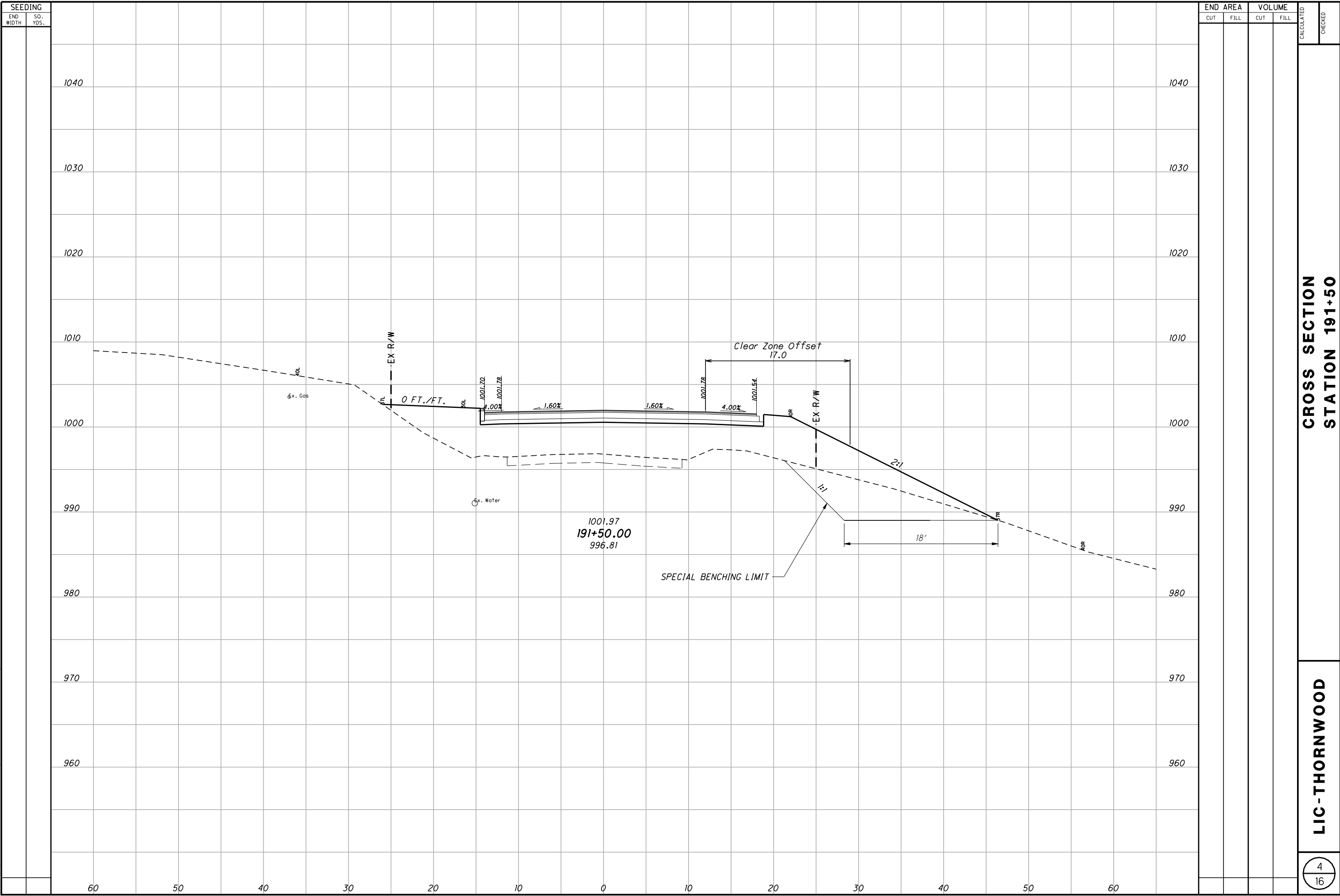
CALCULATED		CHECKED	



**CROSS SECTION  
STATION 191+00**

**LIC-THORWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS004.dgn 10/13/2009 8:35:49 AM dodams



SEEDING	
END WIDTH	SO. YDS.
60	
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

**CROSS SECTION  
STATION 191+50**

**LIC-THORNWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS005.dgn 10/13/2009 8:36:35 AM dadams

SEEDING

END WIDTH	SO. YDS.

END AREA

CUT	FILL

VOLUME

CUT	FILL

CALCULATED

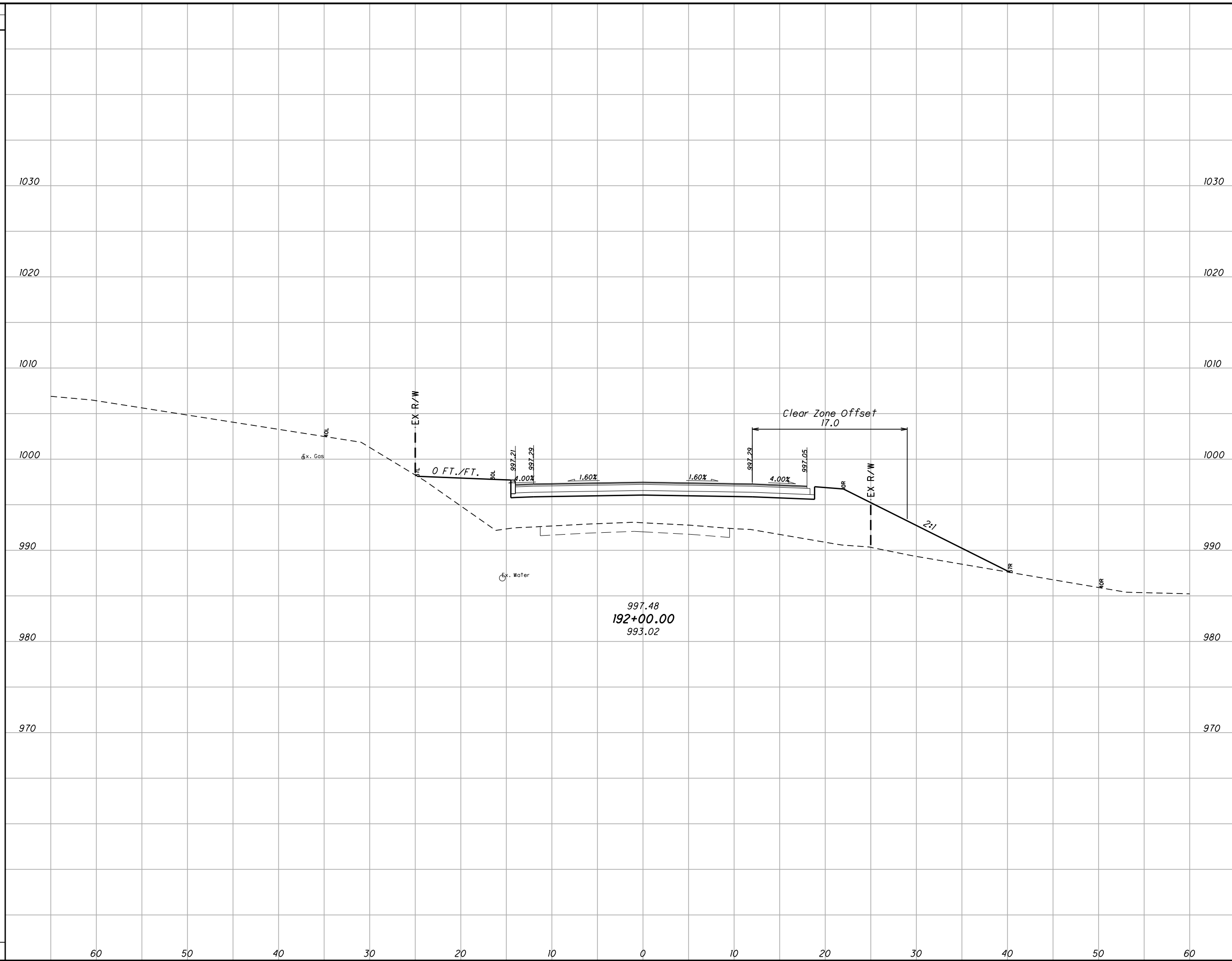
--

CHECKED

**CROSS SECTION  
STATION 192+00**

**LIC-THORNWOOD**

5  
16



60 50 40 30 20 10 0 10 20 30 40 50 60

M:\proj\0821\3004.00\Drawings\Final\78116XS006.dgn 10/13/2009 8:37:12 AM dodams

SEEDING

END WIDTH SQ. YDS.

END AREA

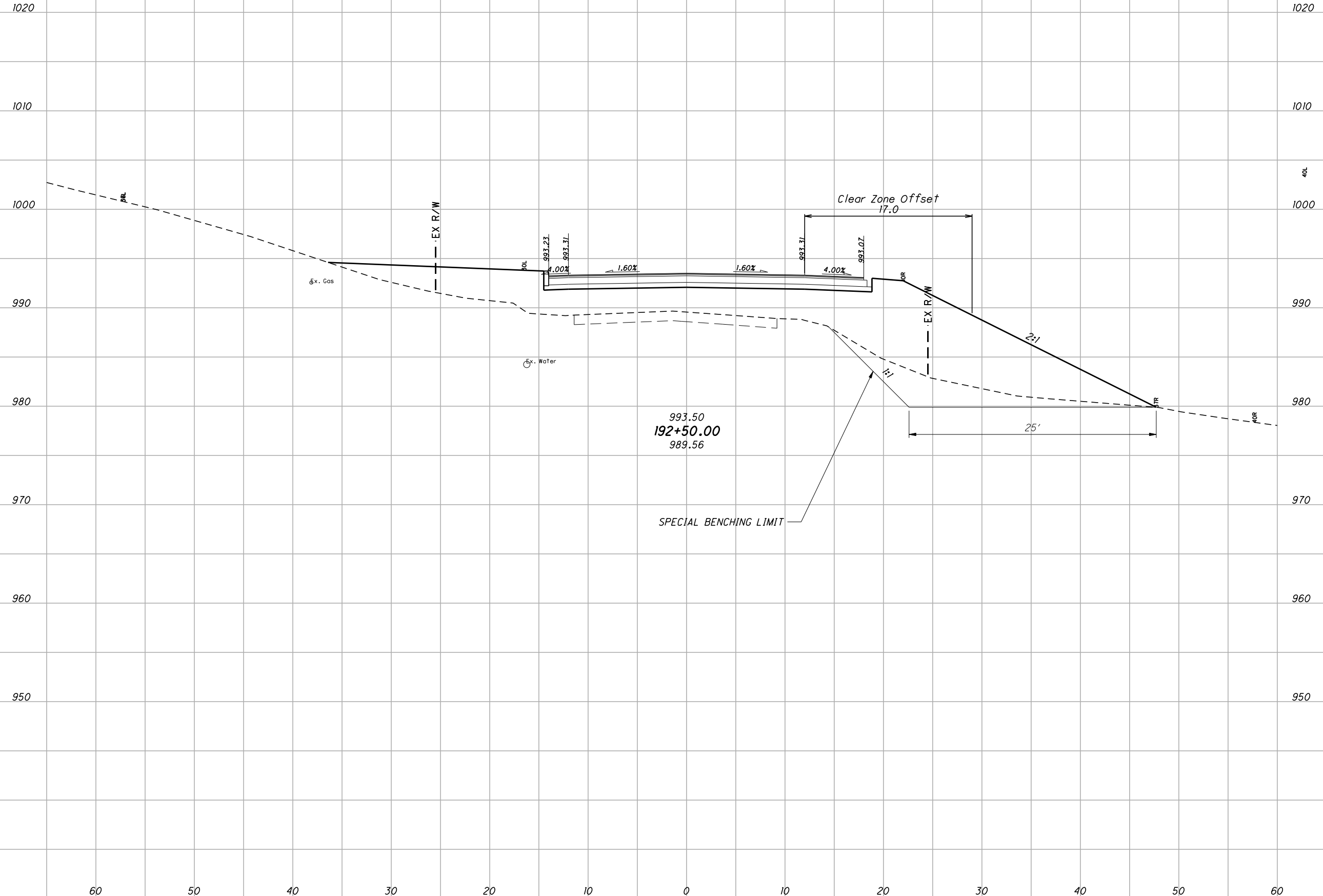
CUT FILL

VOLUME

CUT FILL

CALCULATED

CHECKED



**CROSS SECTION  
STATION 192+50**

**LIC-THORWOOD**

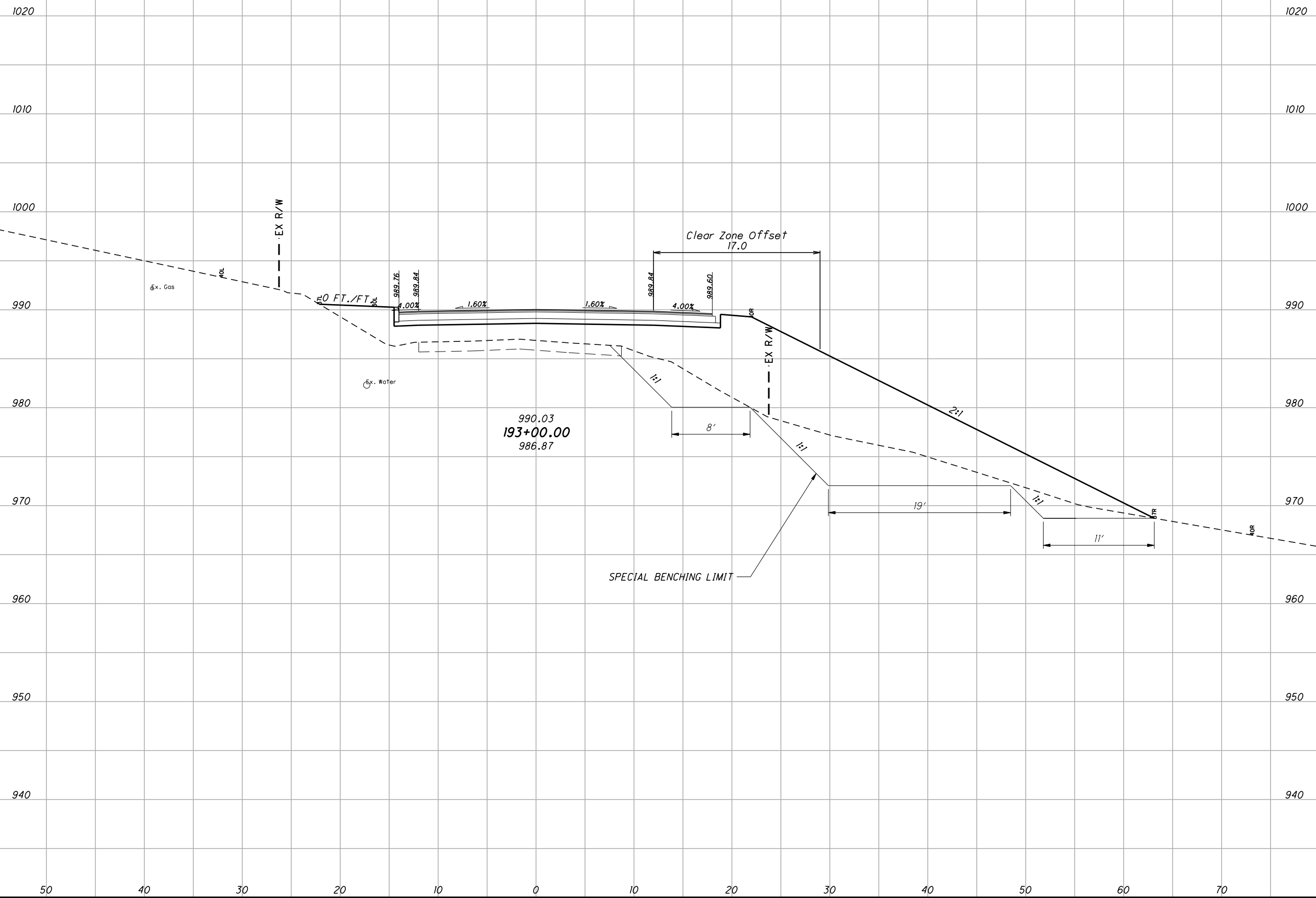
M:\proj\0821\3004.00\Drawings\Final\78116XS007.dgn 10/13/2009 8:37:59 AM dadams

SEEDING

END WIDTH	SO. YDS.
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	
70	

END AREA VOLUME

CUT	FILL	CUT	FILL	CALCULATED	CHECKED



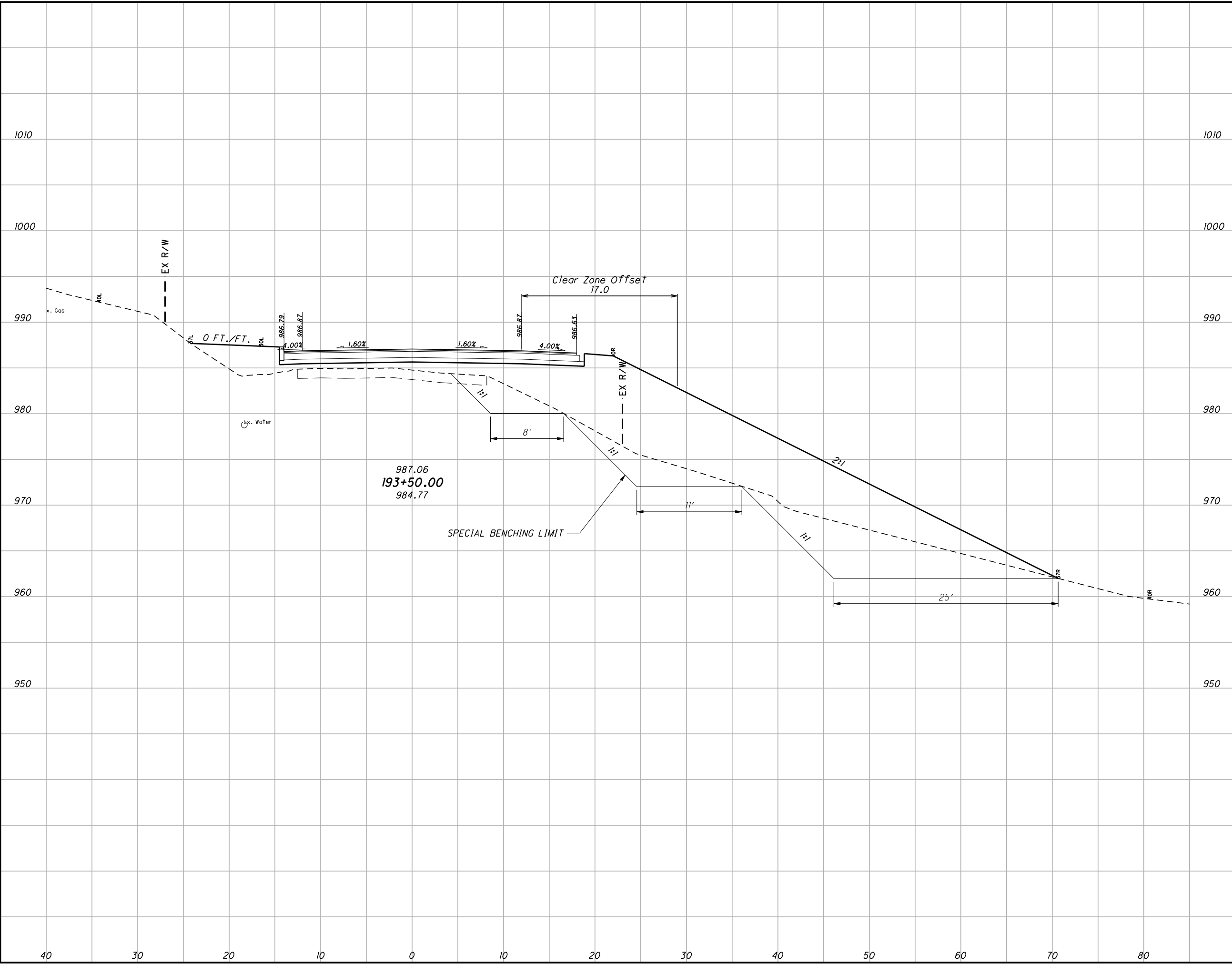
CROSS SECTION  
STATION 193+00

LIC-THORWOOD

M:\proj\0821\3004.00\Drawings\Final\78116XS008.dgn 10/13/2009 8:38:51 AM dodams

SEEDING

END WIDTH	SO. YDS.



END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

**CROSS SECTION  
STATION 193+50**

**LIC-THORNWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS009.dgn 10/13/2009 8:39:34 AM dadams

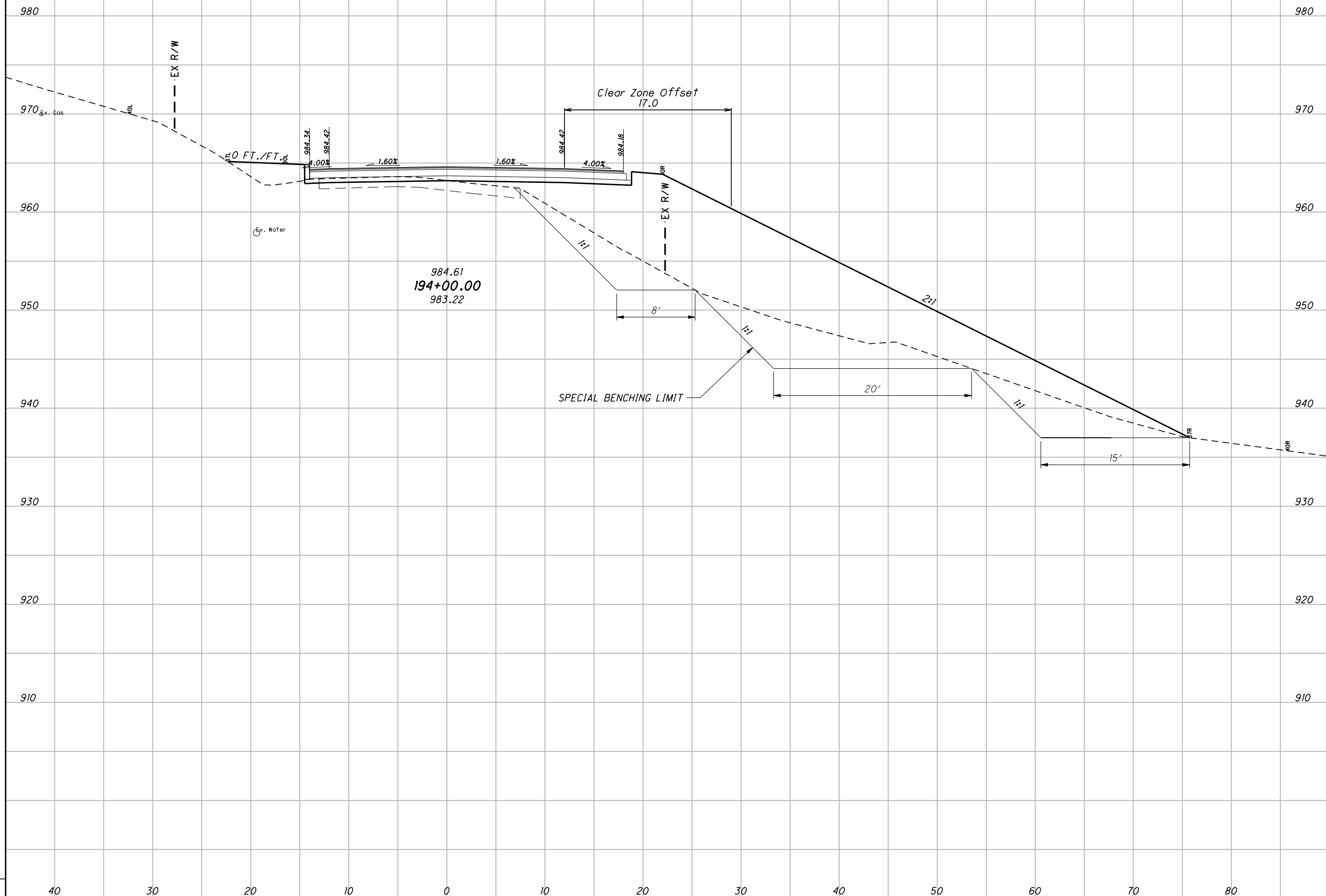
SEEDING	
END WIDTH	SO. YDS.

END AREA		VOLUME	
CUT	FILL	CUT	FILL

CALCULATED	CHECKED
------------	---------

**CROSS SECTION  
STATION 194+00**

**LIC-THORNWOOD**

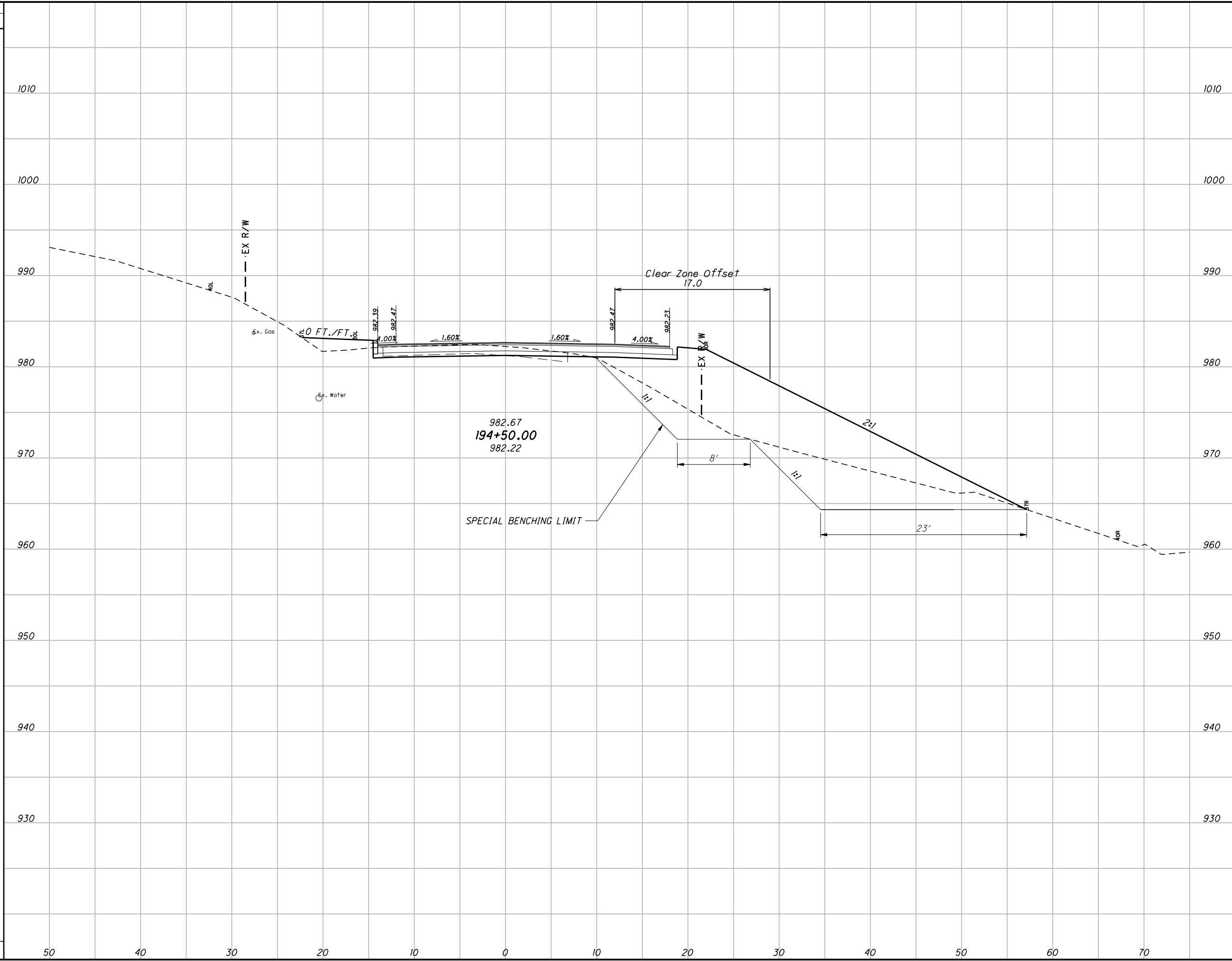


40	30	20	10	0	10	20	30	40	50	60	70	80	90	910	920	930	940	950	960	970	980
----	----	----	----	---	----	----	----	----	----	----	----	----	----	-----	-----	-----	-----	-----	-----	-----	-----

M:\proj\0821\3004.00\Drawings\Final\78116\S010.dgn 10/13/2009 8:44:47 AM dodams

SEEDING

END WIDTH	SO. YDS.
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	
70	



END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

**CROSS SECTION  
STATION 194+50**

**LIC-THORNWOOD**

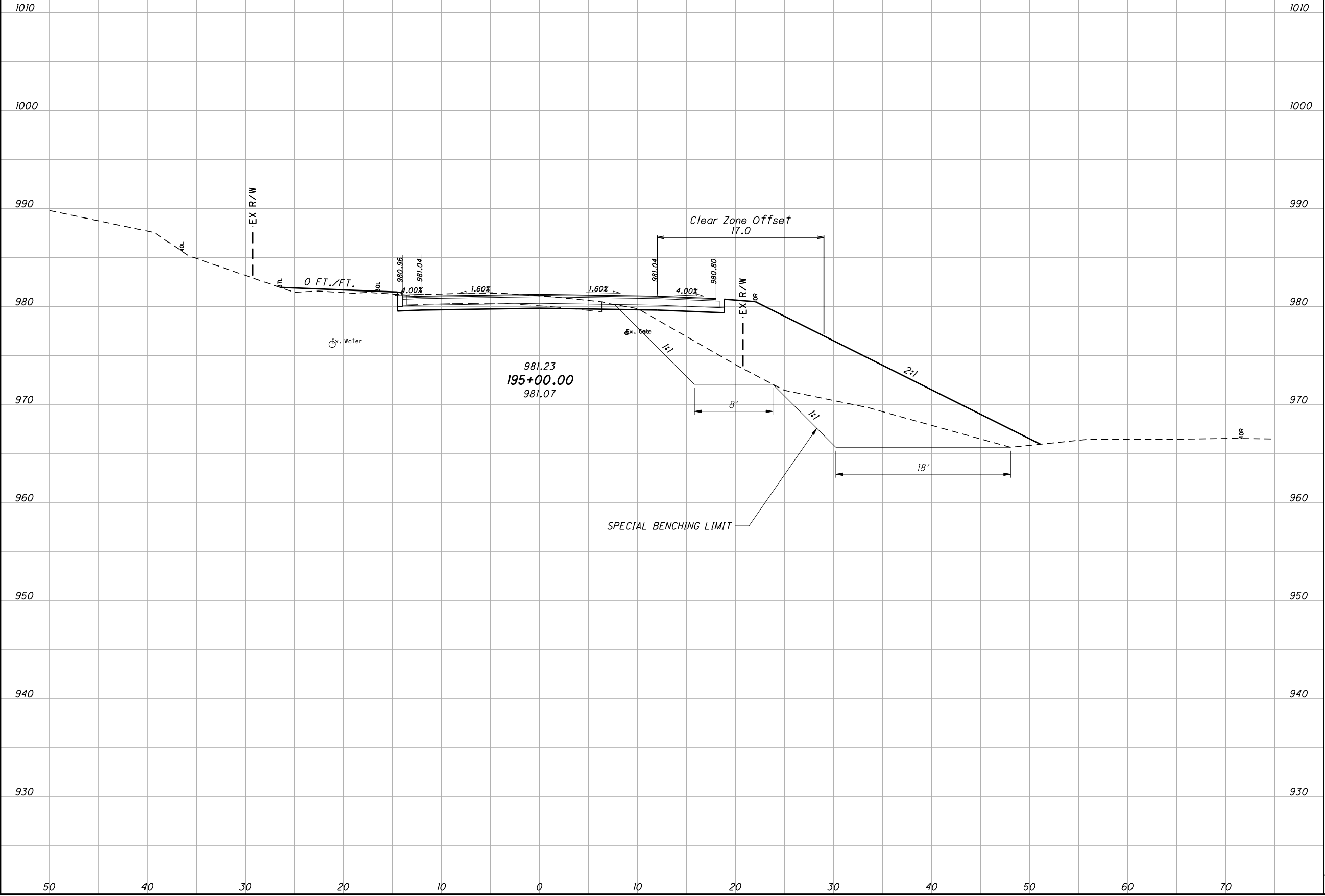


M:\proj\0821\3004.00\Drawings\Final\78116XS011.dgn 10/13/2009 8:46:06 AM dadams

SEEDING  
END WIDTH SQ. YDS.

END AREA VOLUME  
CUT FILL CUT FILL

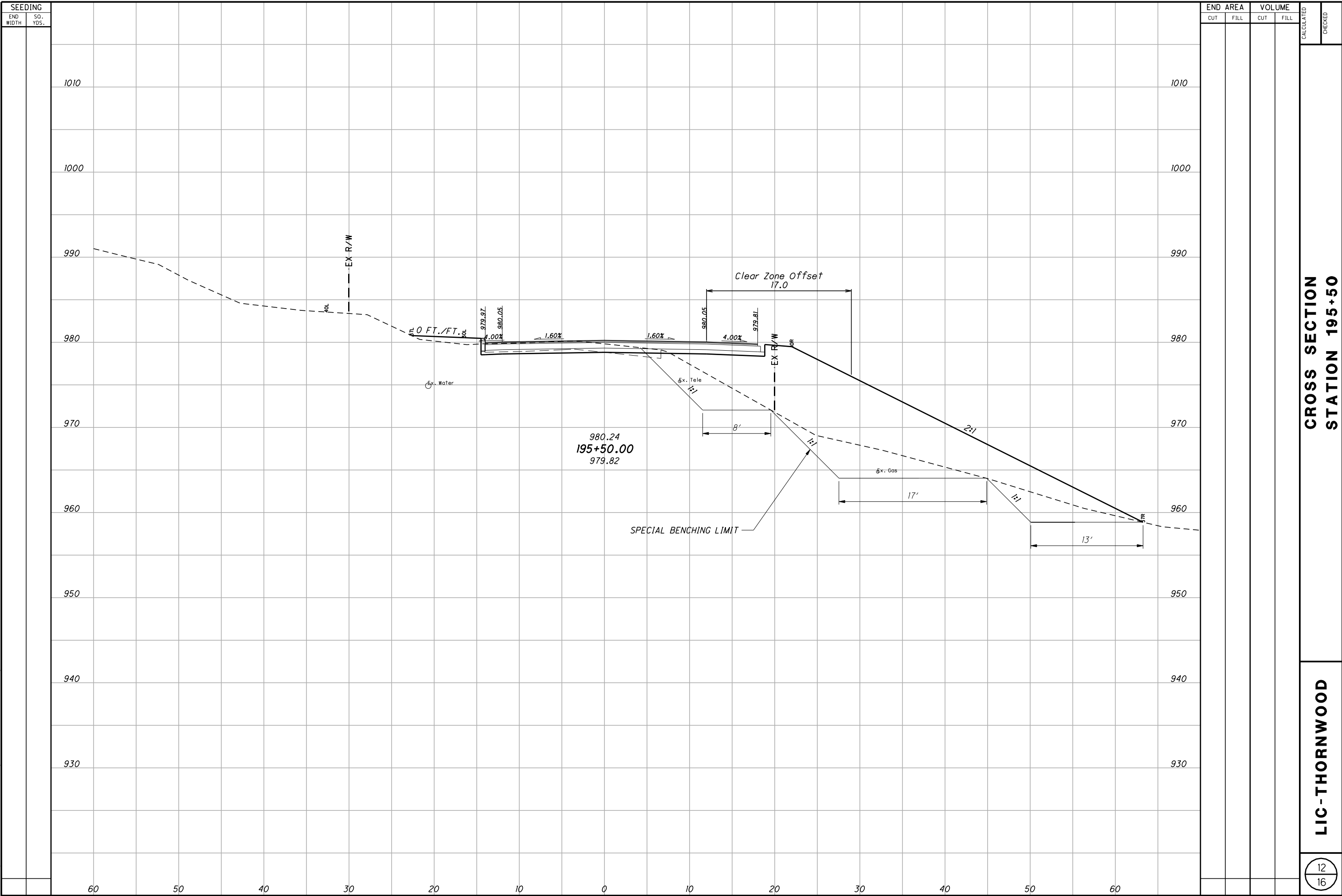
CALCULATED CHECKED



**CROSS SECTION  
STATION 195+00**

**LIC-THORNWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS012.dgn 10/13/2009 8:46:44 AM dodams



SEEDING	
END WIDTH	SO. YDS.

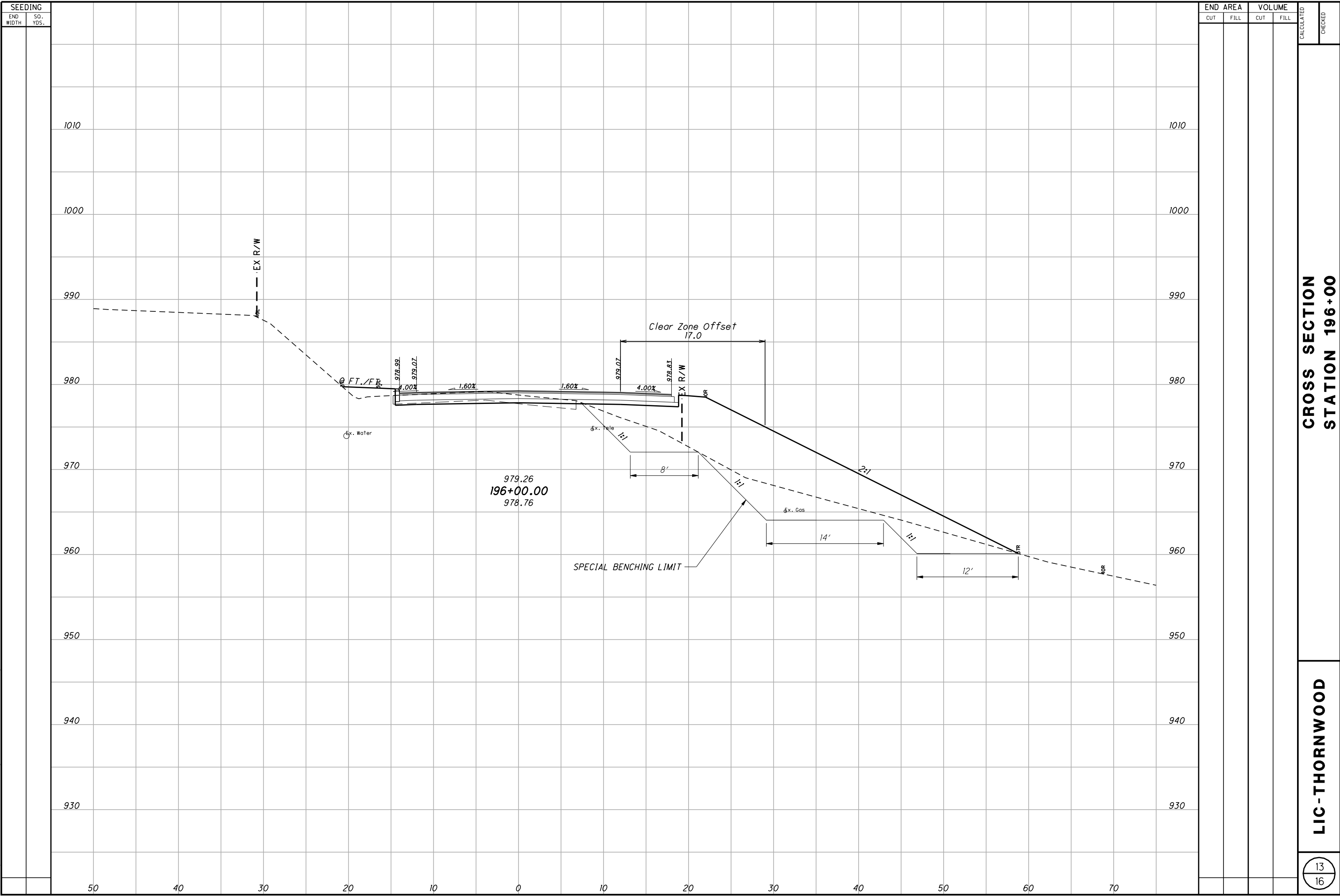
END AREA		VOLUME	
CUT	FILL	CUT	FILL

**CROSS SECTION  
STATION 195+50**

**LIC-THORNWOOD**

12  
16

M:\proj\0821\3004.00\Drawings\Final\78116XS013.dgn 10/13/2009 8:47:31 AM dadams



SEEDING	
END WIDTH	SO. YDS.

END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

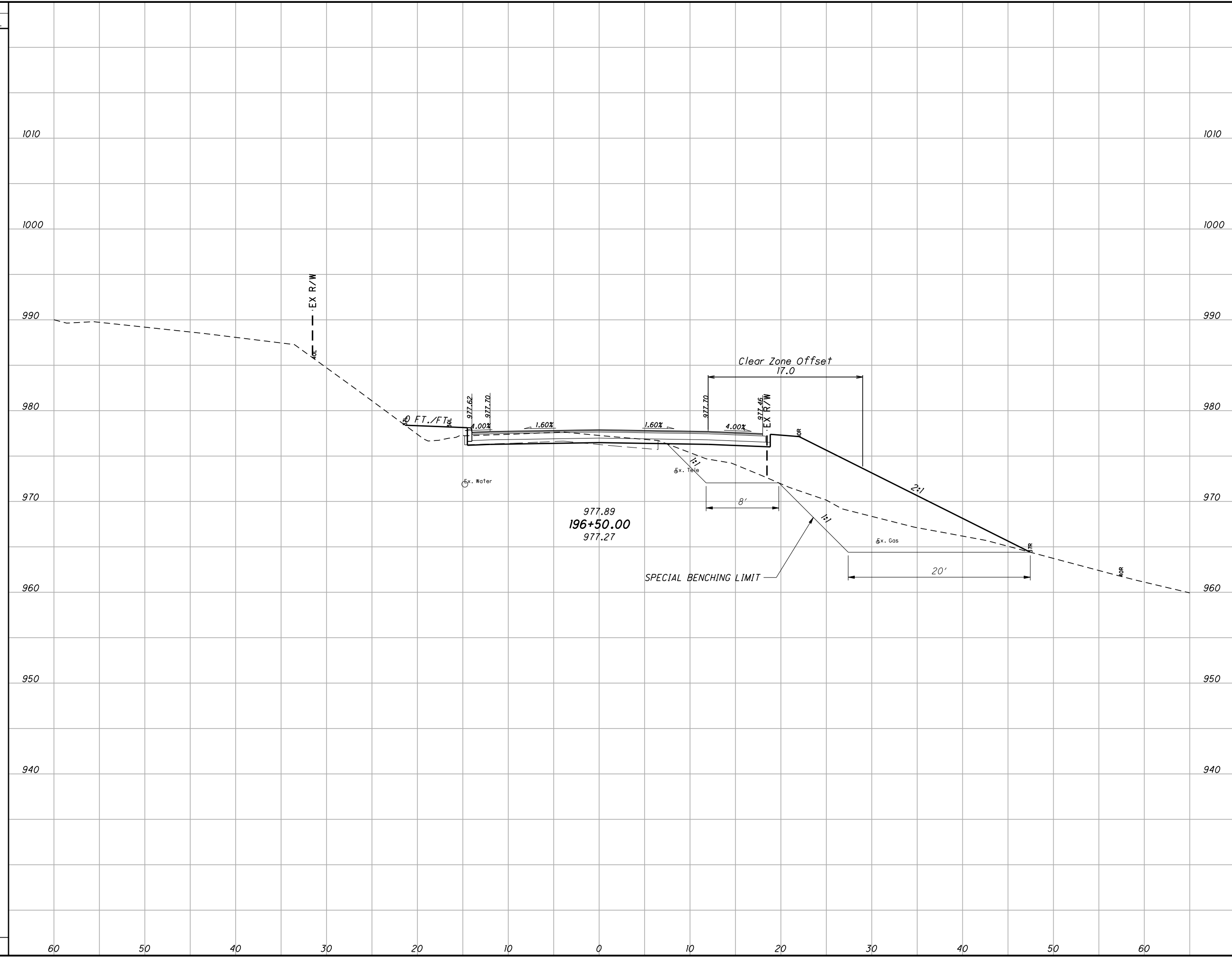
**CROSS SECTION  
STATION 196+00**

**LIC-THORWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS014.dgn 10/13/2009 8:48:31 AM dadams

SEEDING

END WIDTH	SO. YDS.
60	
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	



END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

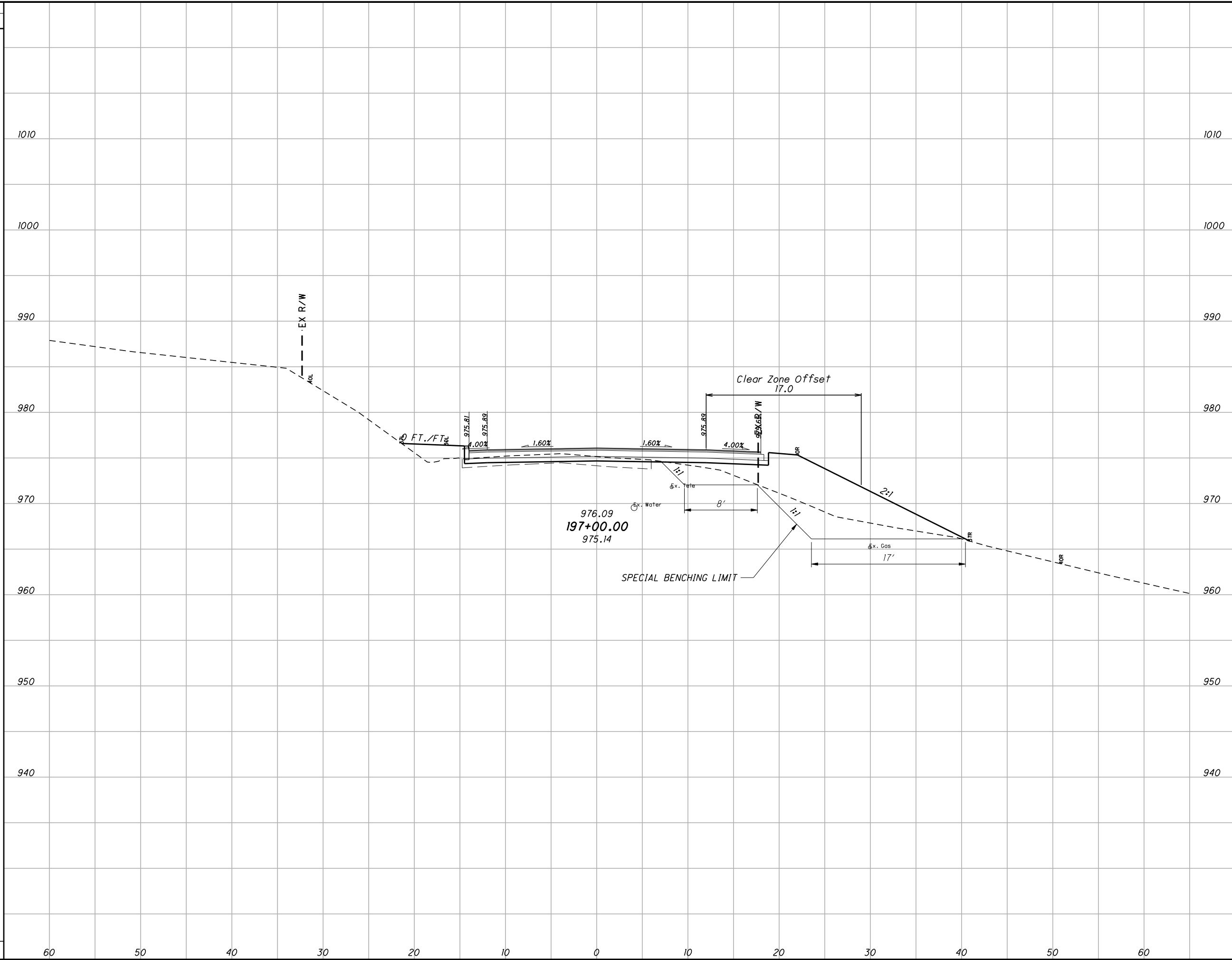
CROSS SECTION  
STATION 196+50

LIC-THORWOOD

M:\proj\0821\3004.00\Drawings\Final\78116XS015.dgn 10/13/2009 9:08:11 AM dadams

SEEDING

END WIDTH	SO. YDS.
60	
50	
40	
30	
20	
10	
0	
10	
20	
30	
40	
50	
60	



END AREA		VOLUME		CALCULATED	CHECKED
CUT	FILL	CUT	FILL		

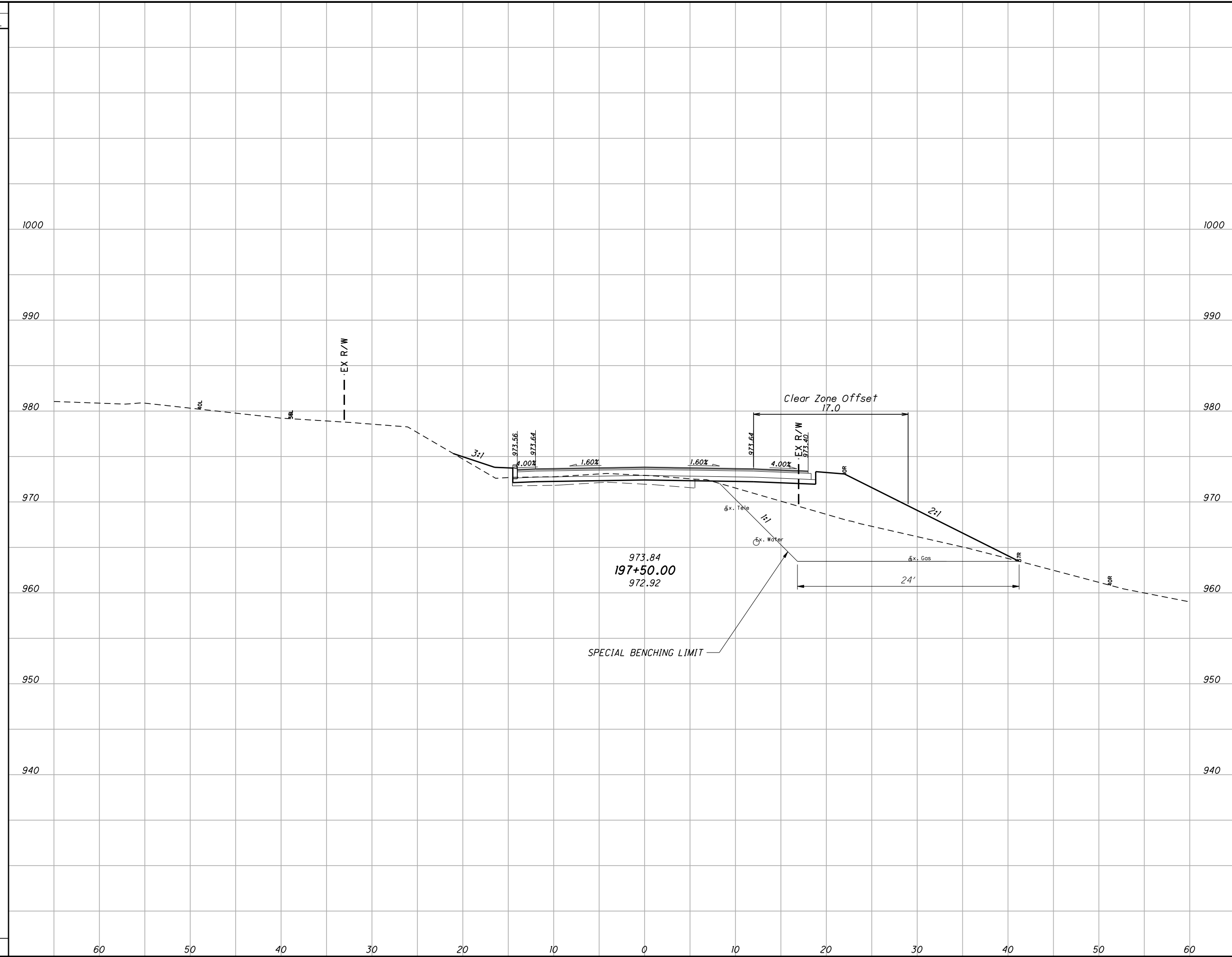
**CROSS SECTION  
STATION 197+00**

**LIC-THORNWOOD**

M:\proj\0821\3004.00\Drawings\Final\78116XS016.dgn 10/13/2009 9:08:50 AM dodams

SEEDING

END WIDTH	SO. YDS.



END AREA

CUT	FILL

VOLUME

CUT	FILL

CALCULATED  
CHECKED

**CROSS SECTION  
STATION 197+50**

**LIC-THORWOOD**