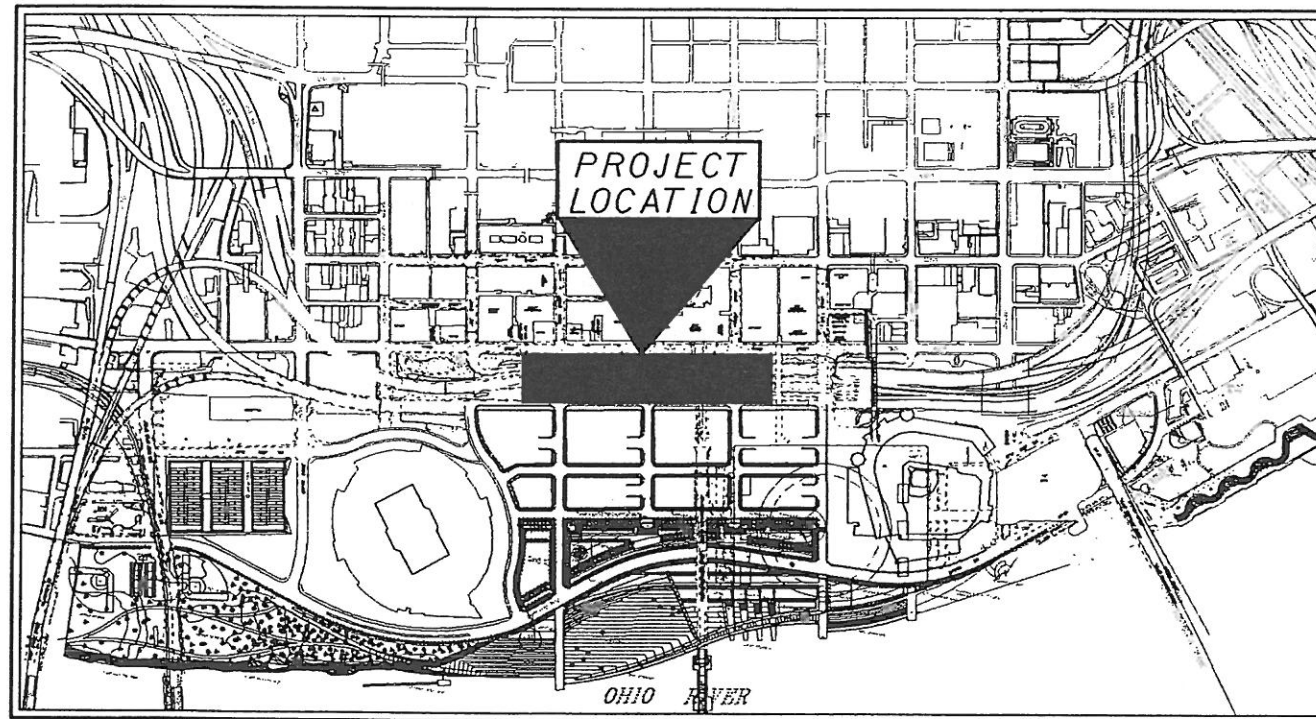


**F O R T
W A S H I N G T O N
W A Y
2 0 0 0**

CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING
PROPOSED IMPROVEMENT
FORT WASHINGTON WAY
RECONSTRUCTION PROGRAM
CONTRACT 26

DECK 1



LOCATION MAP

Plans Reviewed By:



Plans Prepared For:



City of Cincinnati
Engineering Division
Department of Public Works



metric
units

FEDERAL PROJECT NO.

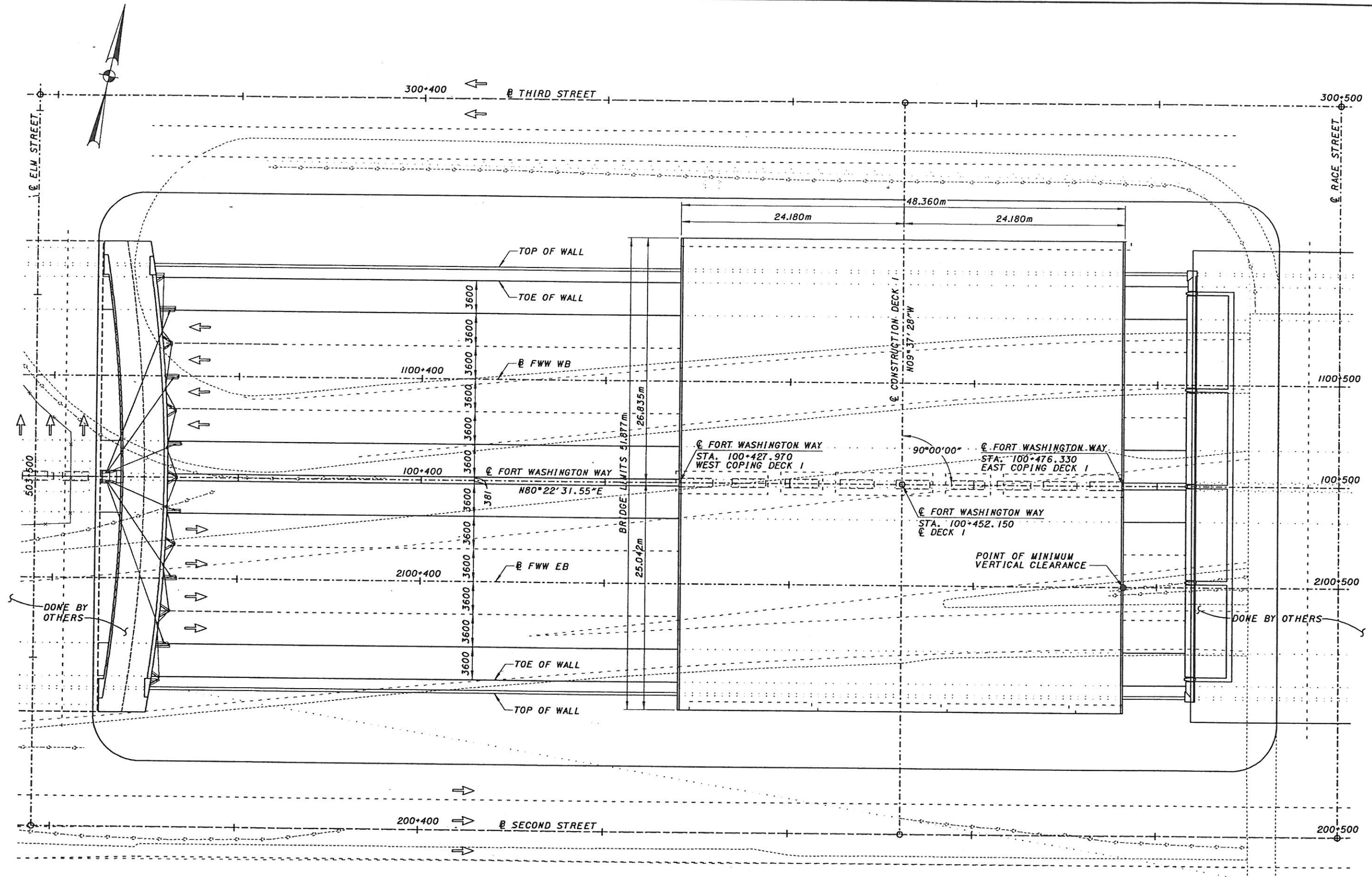
PID NO.

CONSTRUCTION PROJECT NO.

RAILROAD INVOLVEMENT

CITY OF CINCINNATI
CONTRACT No. 75X5753





PLAN

BENCH MARK
SEE GENERAL NOTES
SHEET NO. 13 OF 469
CONTRACT B DRAWINGS

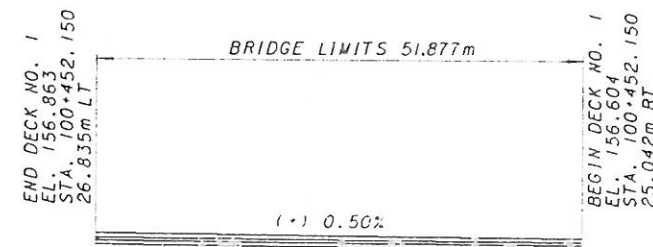
PROPOSED STRUCTURE

TYPE: 2 SPAN PRECAST PRESTRESSED BOX GIRDERS
WITH INTEGRAL ABUTMENTS

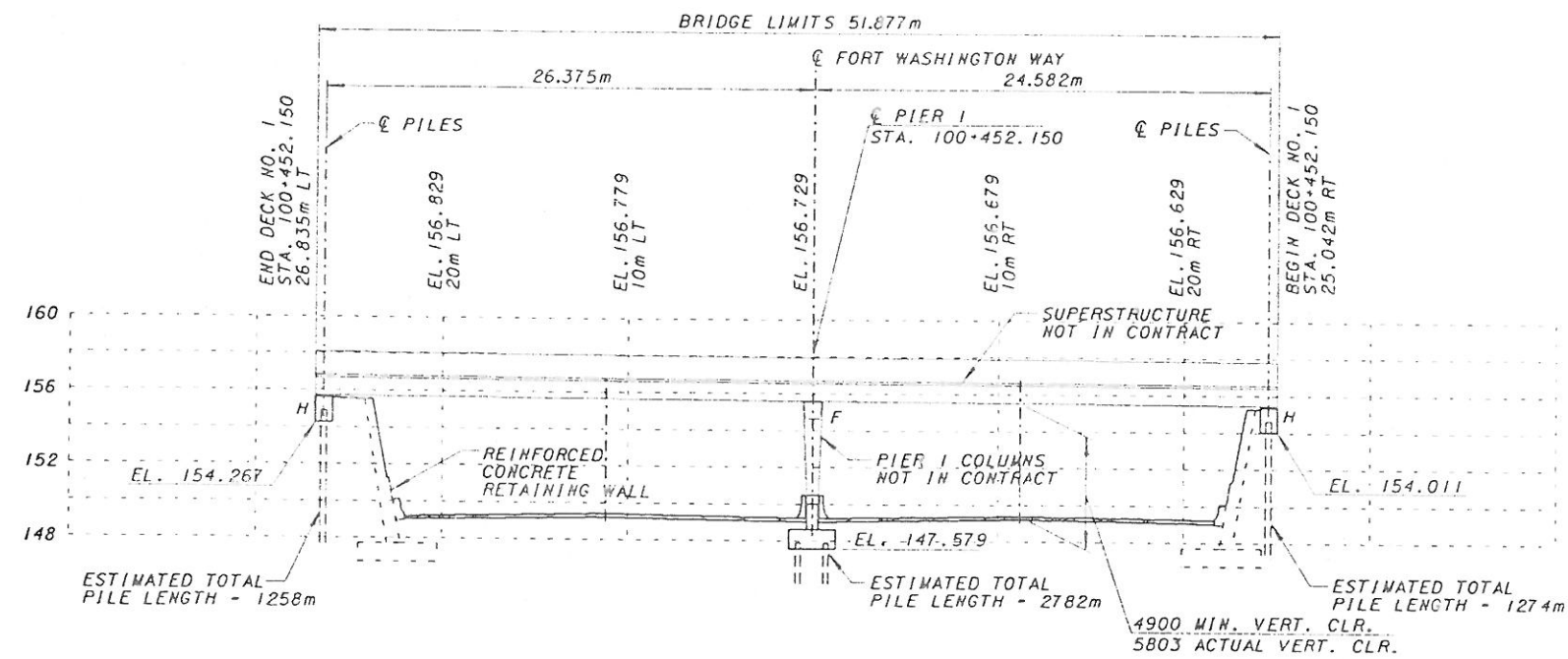
LENGTH OF SPANS: 26.375m - 24.582m
WIDTH OF STRUCTURE: 46.36m
DESIGN LOADING: 1.22m SOIL (MAX DEPTH).
SKEW ANGLE: 0°
SUPER ELEVATION: 0.00356
WEARING SURFACE: NONE
APPROACH SLAB: NOT IN CONTRACT
ALIGNMENT: TANGENT
LATITUDE: N39°-05'-54"
LONGITUDE: W84°-30'-50"

NOTES:

- CONTRACT 26 INCLUDES THE FOLLOWING ITEMS:
- PILES FOR ABUTMENTS AND CENTRAL PIER.
- PILE CAPS FOR ABUTMENTS AND CENTRAL PIER.
- PORTION OF PIER COLUMNS TO 1.4m ABOVE THE TOP OF FOOTING.
- ITEMS NOT INCLUDED IN CONTRACT 24 ARE SHOWN FOR FUTURE REFERENCE TO ASSIST THE FUTURE ENGINEER TO DETERMINE STRUCTURE TYPE ASSUMED IN THE DESIGN OF FOUNDATIONS.



VERTICAL ALIGNMENT ALONG & CONSTRUCTION DECK 1



PROFILE ALONG & CONSTRUCTION DECK 1
(LOOKING UPSTATION)

GENERAL NOTES

STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS

THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS APPLY TO THIS PROJECT:
SUPPLEMENTAL SPECIFICATION 344 : HIGH PERFORMANCE CONCRETE FOR STRUCTURES
SUPPLEMENTAL SPECIFICATION 515 : POST-TENSIONING CONCRETE
SUPPLEMENTAL SPECIFICATION : MASS CONCRETE

DESIGN SPECIFICATIONS

THIS PROJECT IS DESIGNED IN METRIC SI UNITS. ALL DIMENSIONS ARE IN MILLIMETERS (mm), AND ALL ELEVATIONS ARE IN METERS (m), UNLESS NOTED OTHERWISE.

THIS STRUCTURE CONFORMS TO

"STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, SIXTEENTH EDITION, 1996, INCLUDING THE 1997 INTERIM SPECIFICATIONS

STATE OF OHIO "BRIDGE DESIGN MANUAL"

STATE OF OHIO "CONSTRUCTION AND MATERIAL SPECIFICATIONS", 1997.

DESIGN LOADINGS

DEAD LOADS

UNIT WEIGHT OF SOIL IN PLANTERS - 19.1N/m³
MAXIMUM HEIGHT OF SOIL - 1.22 METER

UNIT WEIGHT OF REINFORCED CONCRETE - 23.6 kN/m³

WEIGHT OF THE PRESTRESSED CONCRETE BOX BEAMS INCLUDING
150 mm DECK - 13.67 kN/m³

ADDITIONAL SUPERIMPOSED DEAD LOAD - 1.8 kN/m²

SNOW LOAD

SNOW LOADING - 0.72 kN/m²

THERMAL FORCES

ANTICIPATED THERMAL MOVEMENTS AND FORCES WERE CALCULATED BASED ON THE FOLLOWING:

DESIGN MEAN TEMPERATURE - 21 DEG. C (68 DEG. F.)
TEMPERATURE RISE - 19 DEG. C (35 DEG. F.)
TEMPERATURE FALL - 25 DEG. C (45 DEG. F.)
COEFFICIENT OF LINEAR EXPANSION - 0.0000108 PER DEGREE C.

SEISMIC FORCES

SITE HORIZONTAL ACCELERATION- 0.06g. DESIGN DETAILS IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SIXTEENTH EDITION, SEISMIC PERFORMANCE CATEGORY A AND ODOT BDM, 3.1.2.1.

LOADING COMBINATIONS

LOAD FACTORS, ALLOWABLE OVERSTRESS AND LOADING COMBINATIONS ARE IN ACCORDANCE WITH AASHTO TABLE 3.2.2.1A.

CLEARANCE

A MINIMUM VERTICAL CLEARANCE BETWEEN THE RIDING SURFACE OF ROADWAY BELOW AND THE LOWEST POINT OF THE STRUCTURE ABOVE, IF IT ENCLOSED ONTO TRAFFIC LANES OF ROADWAY BELOW, SHALL BE 4.3 m FOR MAINLINE ROADWAYS, RAMPS AND LOCAL STREETS.

CONCRETE

MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'c) SHALL BE AS FOLLOWS:
HIGH PERFORMANCE CONCRETE - COMPRESSIVE STRENGTH 38.0 MPa TO BE USED FOR PIER COLUMNS.

CONCRETE CLASS "C" - COMPRESSIVE STRENGTH 27.0 MPa TO BE USED FOR FOUNDATIONS AND ABUTMENTS.

CONSTRUCTION JOINTS IDENTIFIED ON THE DRAWINGS AS "ROUGHENED", SHALL BE CLEAN AND SHALL BE ROUGHENED TO A FULL AMPLITUDE OF APPROX. 6 mm.

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615M, A616M OR A617M, GRADE 420 WITH MINIMUM YIELD STRENGTH OF 420 MPa.

ALL REINFORCING STEEL SHALL BE EPOXY COATED.

MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE SHOWN ON THE DRAWINGS. CONCRETE COVERS SHOWN ON THE DRAWINGS DO NOT INCLUDE PLACEMENT AND FABRICATION TOLERANCES, UNLESS SHOWN AS A "MINIMUM COVER".

DESIGN METHOD

POST-TENSIONED CONCRETE MEMBER SHALL BE DESIGNED IN ACCORDANCE WITH AASHTO 9.13.1.2.

REINFORCING CONCRETE MEMBERS SHALL BE DESIGNED USING THE STRENGTH DESIGN METHOD.

PROTECTION OF TRAFFIC DURING DEMOLITION

PRIOR TO THE DEMOLITION OF THE EXISTING STRUCTURE, THE CONTRACTOR SHALL SUBMIT HIS PLANS FOR THE PROTECTION OF TRAFFIC.

END BEARING PILES DRIVEN TO BEDROCK

THE ABUTMENT AND PIER PILES SHALL BE DRIVEN INTO BEDROCK A MINIMUM OF 300 mm OR TO PRACTICAL REFUSAL ON BEDROCK. REFUSAL SHALL BE CONSIDERED AS OBTAINED BY PENETRATING SOFT BEDROCK FOR SEVERAL MILLIMETERS AND HAVING A RESISTANCE OF 20 BLOWS PER 25 mm OR REFUSAL SHALL BE CONSIDERED AS OBTAINED AFTER THE PILE HAS PENETRATED BEDROCK AND THE PILE HAS THEN RECEIVED AT LEAST 20 BLOWS.

PILE LOADS - THE DESIGN ALLOWABLE CAPACITY FOR HP 12x53 PILES IS 900 kN IN COMPRESSION.

PILES NOT BEARING ON BEDROCK WILL NOT BE ACCEPTED. AUGER SOUNDINGS MAY BE REQUIRED IF THE PILES ENCOUNTER REFUSAL SIGNIFICANTLY ABOVE DESIGN ELEVATIONS.

PILE HAMMER

THE PILE HAMMER USED TO INSTALL THE PILES SHALL HAVE A STATE'S ENERGY RATING OF NOT LESS THAN 54000 JOULES. THIS REQUIREMENT DOES NOT RELIEVE THE CONTRACTOR FROM 102.05 WHICH STATES THAT THE CONTRACTOR IS TO PROVIDE SUFFICIENT EQUIPMENT FOR PROSECUTING THE REQUIRED WORK. REFER TO "ODOT'S MANUAL OF PROCEDURES FOR STRUCTURES" TO OBTAIN THE STATE'S ENERGY RATING.

STEEL POINTS

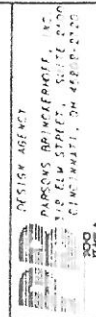
STEEL POINTS SHALL BE USED TO PROTECT THE TIPS OF THE PROPOSED "H" PILING. THE STEEL POINTS SHALL BE FURNISHED BY THE ASSOCIATED PILE AND FITTING CORPORATION, 262 RUTHERFORD BOULEVARD, CLIFTON, NEW JERSEY 07014; INTERNATIONAL CONSTRUCTION EQUIPMENT, INC., 301 WAREHOUSE DRIVE, MATHEWS, NORTH CAROLINA 28015; OR BY A MANUFACTURER THAT CAN FINISH A STEEL POINT THAT IS ACCEPTABLE TO THE ENGINEER.

TEST PILES

THE LOCATION OF THE TEST PILE SHALL BE DETERMINED BY THE ENGINEER. THE TEST PILE SHALL BE SUBJECTED TO DYNAMIC LOAD TESTING IN ACCORDANCE WITH CMS 523.

PRODUCTION PILES

AS DIRECTED BY THE ENGINEER, PRODUCTION PILES SHALL BE DYNAMICALLY TESTED IN ACCORDANCE WITH CMS 523. THE NUMBER OF PILES AND WHICH PILES TO BE TESTED WILL BE DETERMINED BY THE ENGINEER. THE DYNAMIC TESTING SHALL BE PERFORMED DURING BOTH THE INITIAL AND RETRAIL DRIVING. THE DYNAMIC TESTING EQUIPMENT SHOULD CONFORM TO THE REQUIREMENTS OF ASTM D-4945-89, STANDARD TEST METHOD FOR HIGH STRAIN DYNAMIC TESTING OF PILES.



DESIGN AGENCY
PARSONS BRINCKERHOFF, INC.
110 ELM STREET, SUITE 2100
CINCINNATI, OH 45202-2020

DATE
6-30-00
PROJECT FILE NUMBER
100-427-623

LOCATION NAME
STA. 100+427.623
STA. 100+477.000

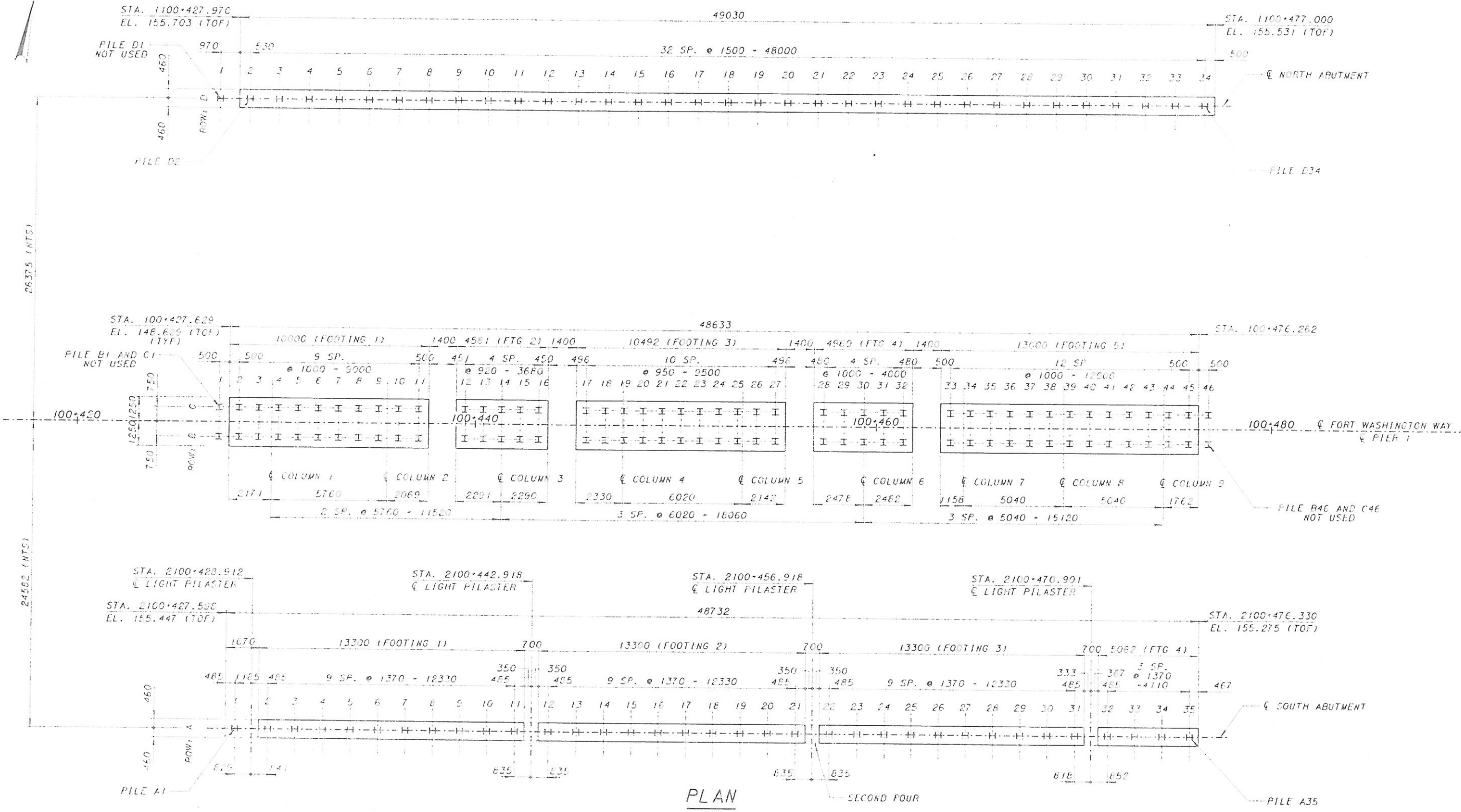
GENERAL NOTES
DECK NUMBER 1
OVER FORT WASHINGTON WAY

CITY OF CINCINNATI
CONTRACT NO. 75X5753

3/19

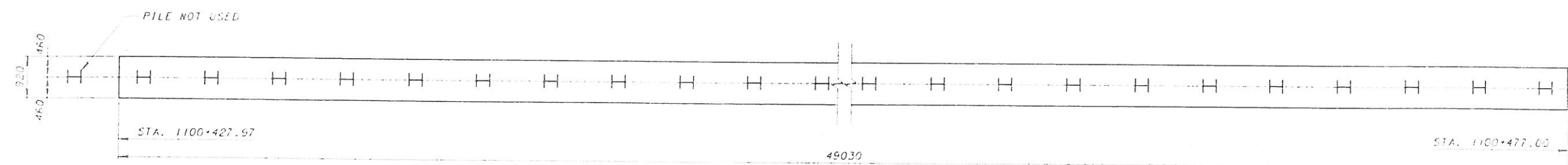


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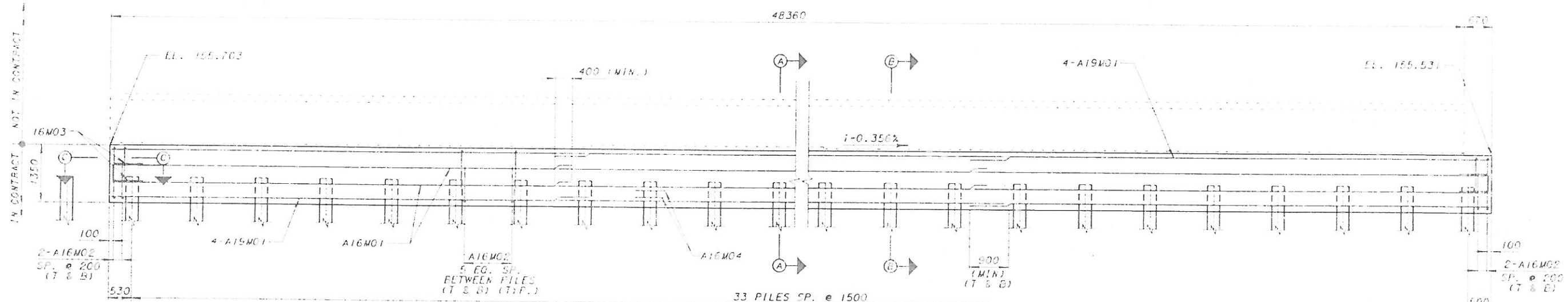


LEGEND
H-HP 12 X 53 STEEL PILE (TYP)
DRIVEN TO ROCK
980 KN PILE CAPACITY

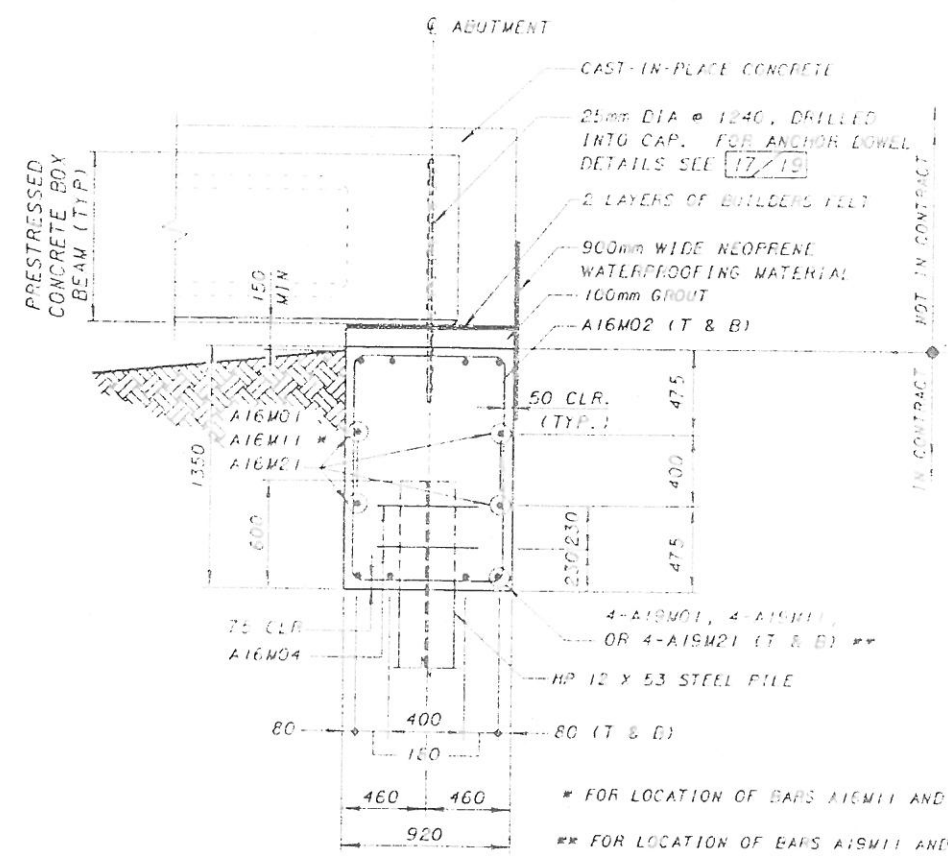
	DESIGN AGENCY	HAMILTON COUNTY STA. 100+427.529 STA. 100+477.000	FOUNDATION LAYOUT DECK NUMBER 1 OVER FORT WASHINGTON WAY	CITY OF CINCINNATI CONTRACT NO. 75X5753
	DATE 6-30-00			
	REVISED STRUCTURE FILE NUMBER			
	DRAWN SRL			
PREPARED VJR	CHECKED JSH	4/19		



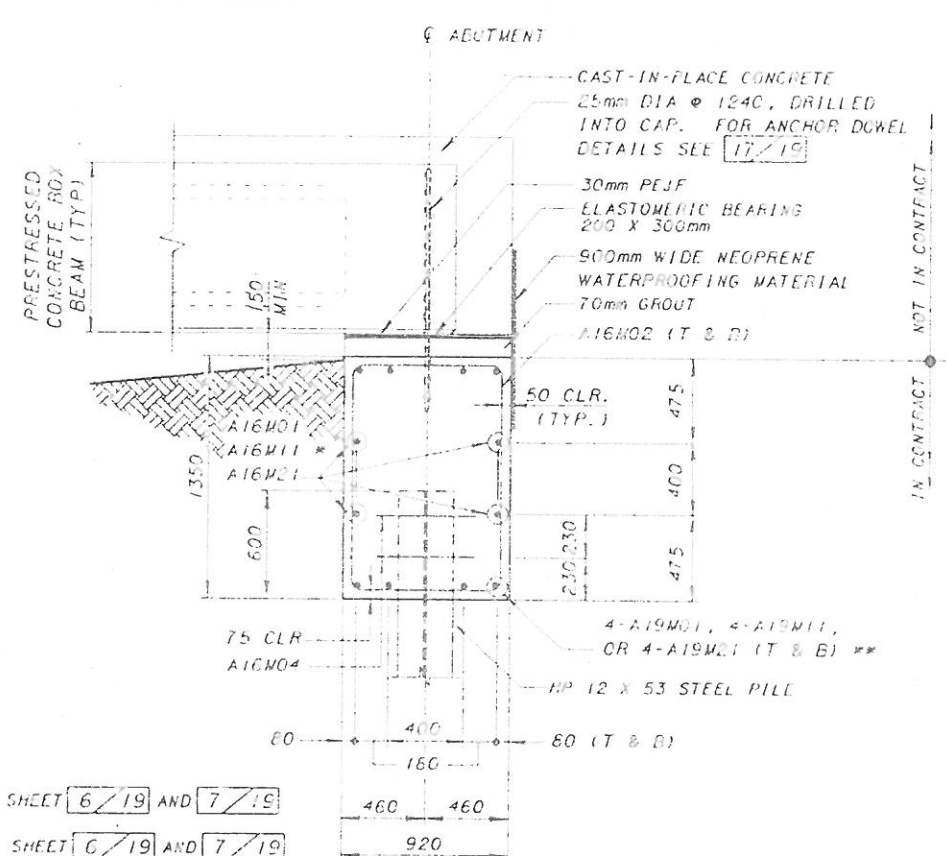
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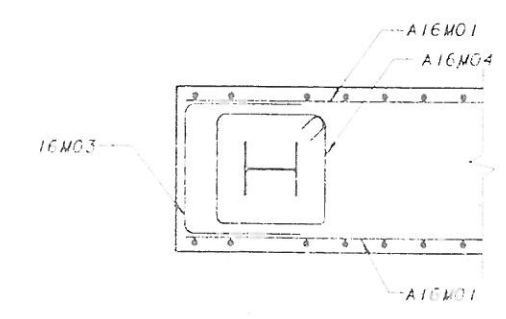
ELEVATION



SECTION A-A
(FIXED CONNECTION)

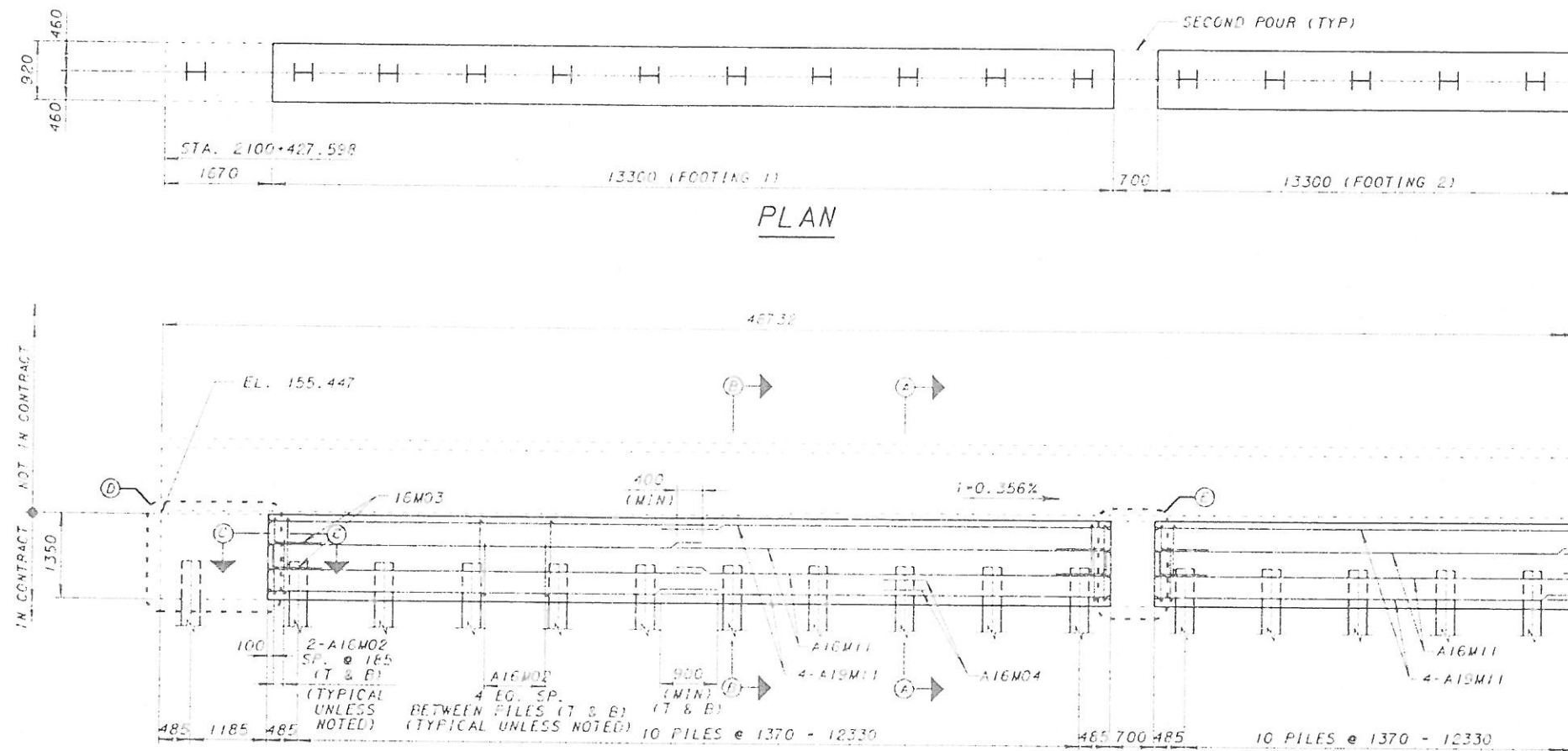


SECTION B-B
(EXPANSION CONNECTION)



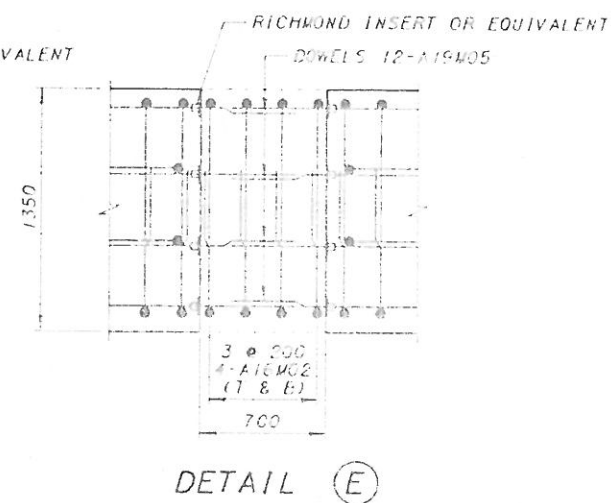
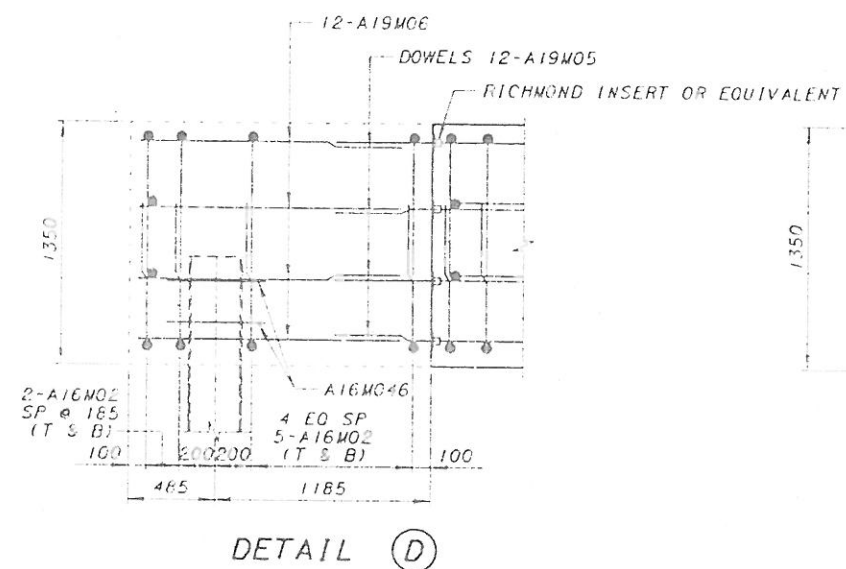
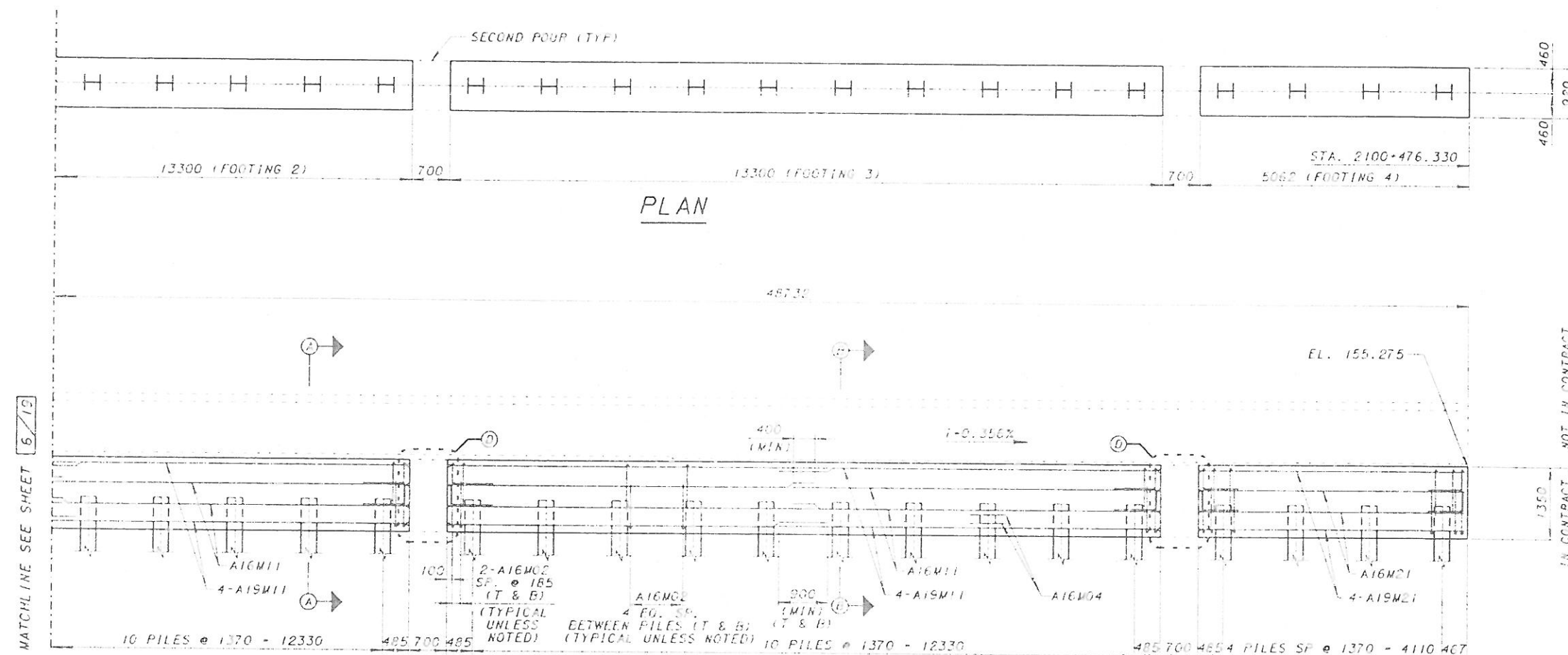
SECTION C-C

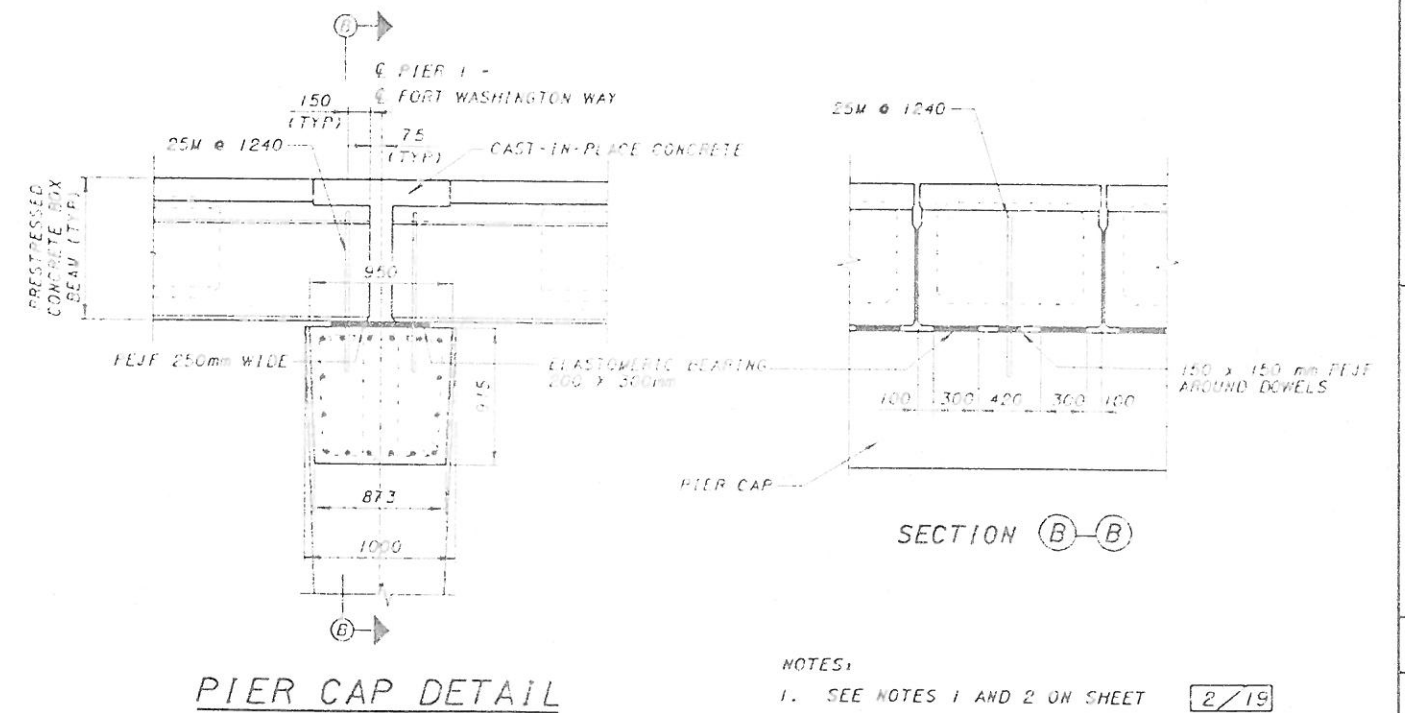
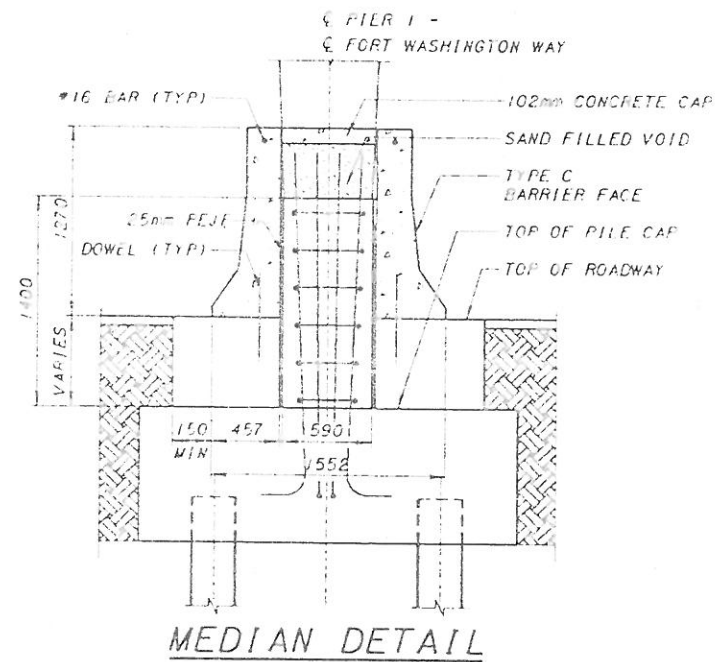
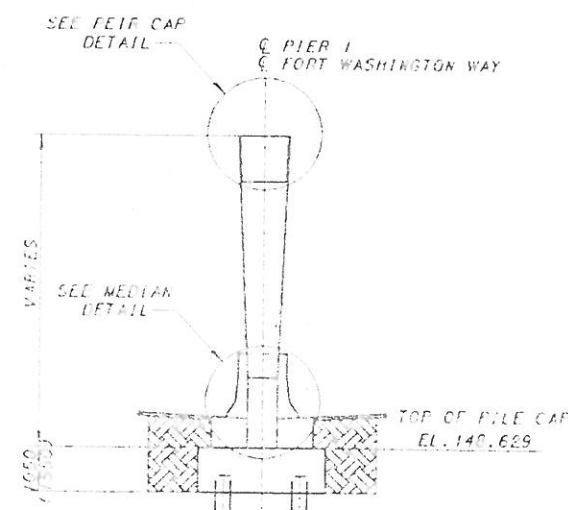
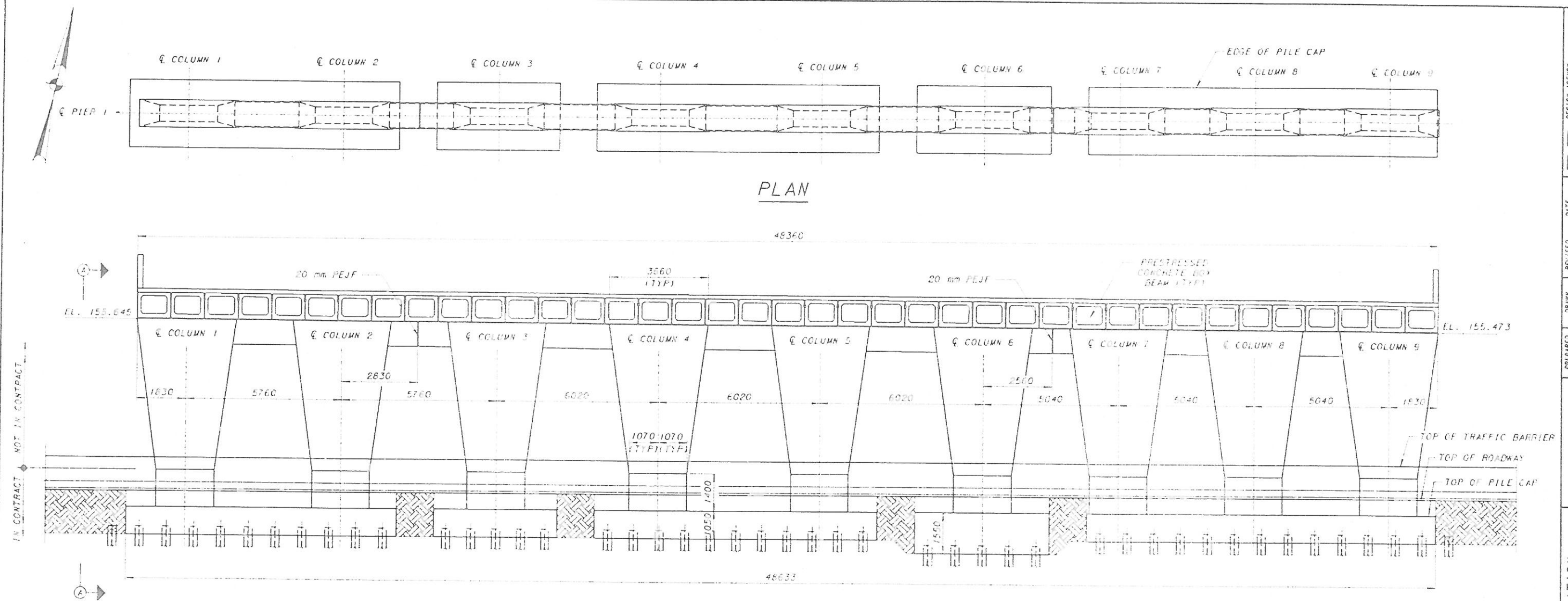
* FOR LOCATION OF BARS A16M11 AND A16M21 SEE SHEET 6/19 AND 7/19
 ** FOR LOCATION OF BARS A16M11 AND A16M21 SEE SHEET 6/19 AND 7/19



ELEVATION

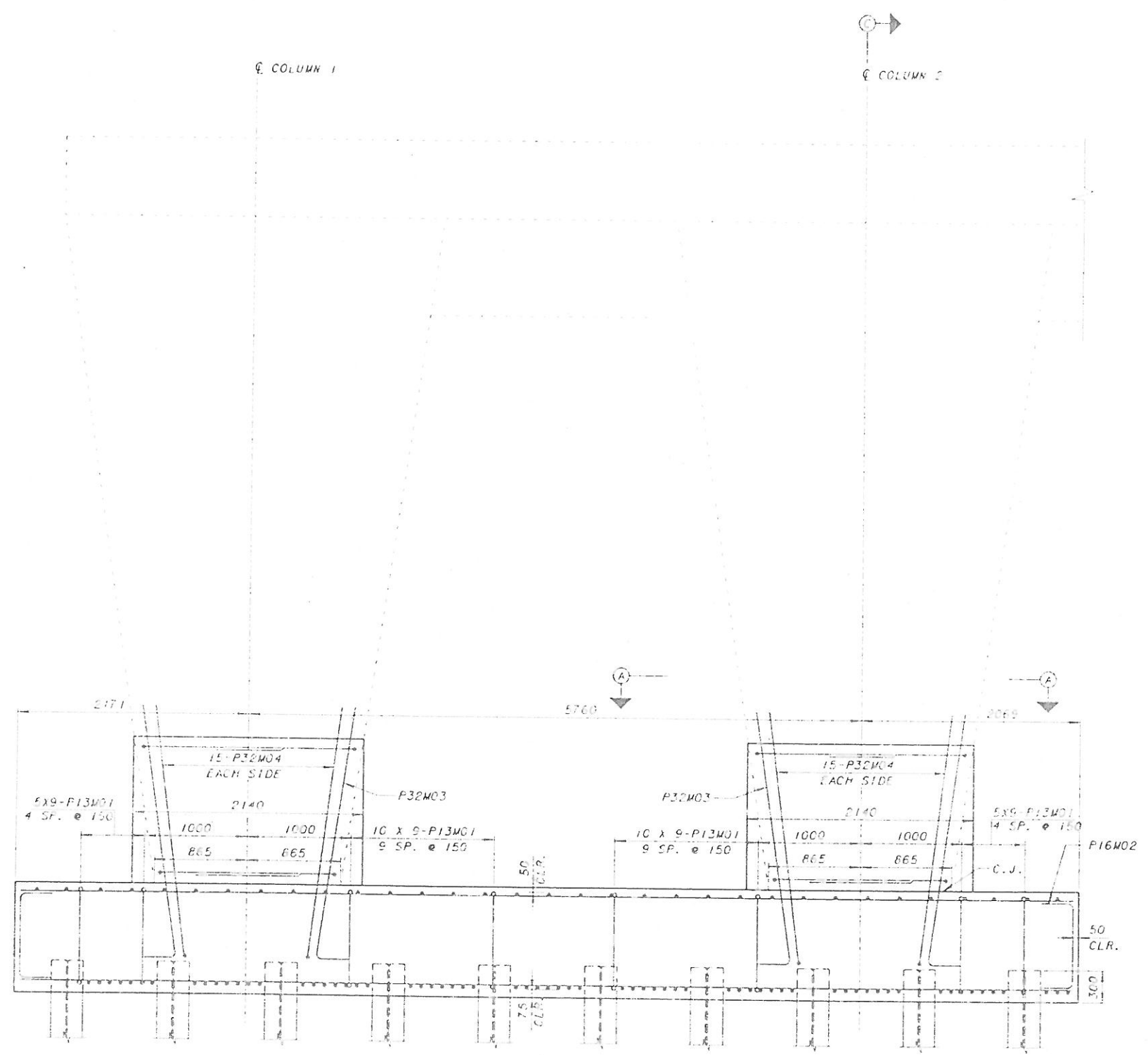
NOTES:
1. FOR SECTIONS A-A, B-B, AND C-C SEE SHEET 5/19



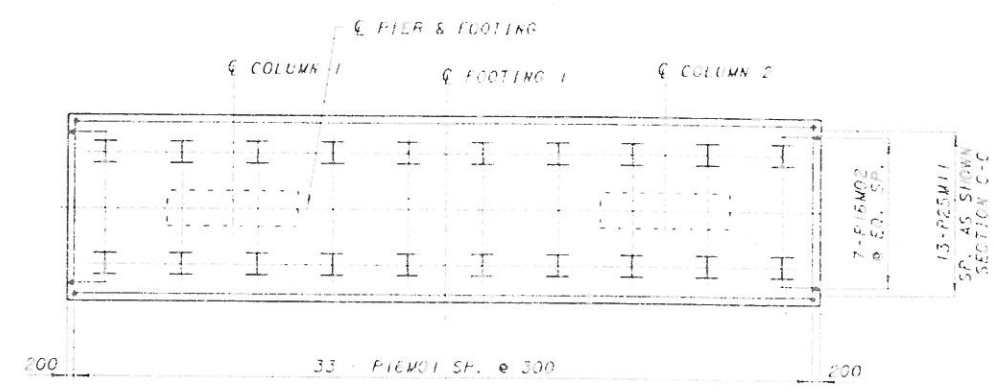
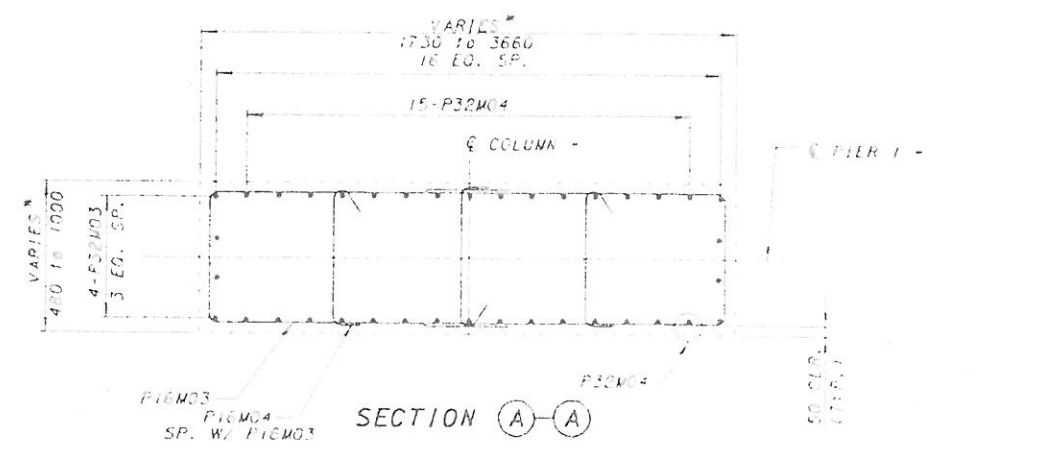


NOTES:
1. SEE NOTES 1 AND 2 ON SHEET 2/19

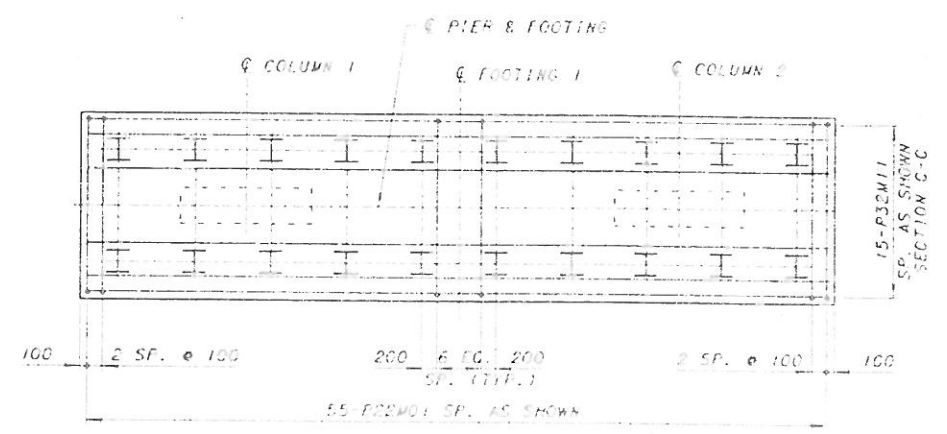
FILE NO. 100-427-629
SHEET NO. 10/19
DATE 5-30-00
REVISED 5-30-00
DESIGNED BY JSH
CHECKED BY JSH
DRAWN BY JSH
REVIEWED BY JSH
DATE 5-30-00
PROJECT NO. 100-427-629
HAMILTON COUNTY
STA. 100+427.629
STA. 100+477.000
PIER 1 REINFORCEMENT (1 OF 5)
DECK NUMBER 1
OVER FORT WASHINGTON WAY
CITY OF CINCINNATI
CONTRACT NO. 75X5753
9/19



SECTION B-B
SEE SHEET 10/19 FOR LOCATION OF SECTION B-B



PLAN FOOTING 1 TOP REINFORCING

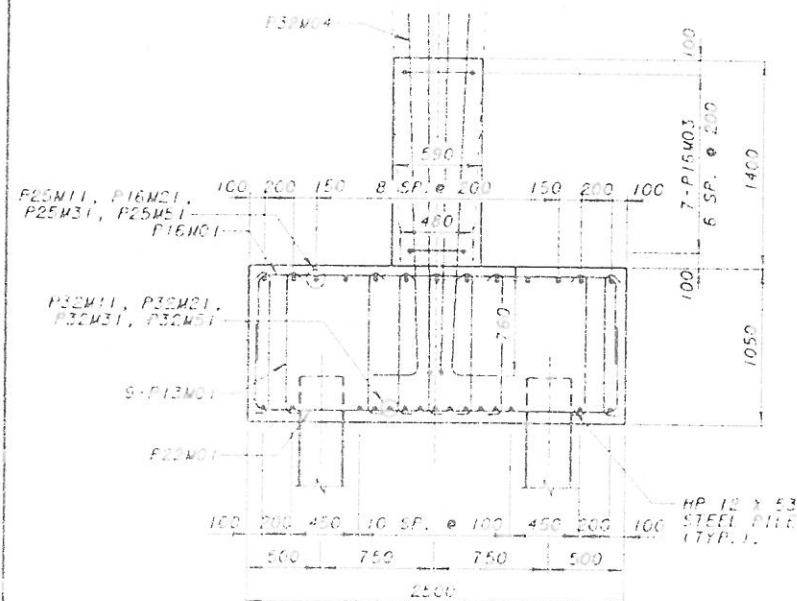


PLAN FOOTING 1 BOTTOM REINFORCING

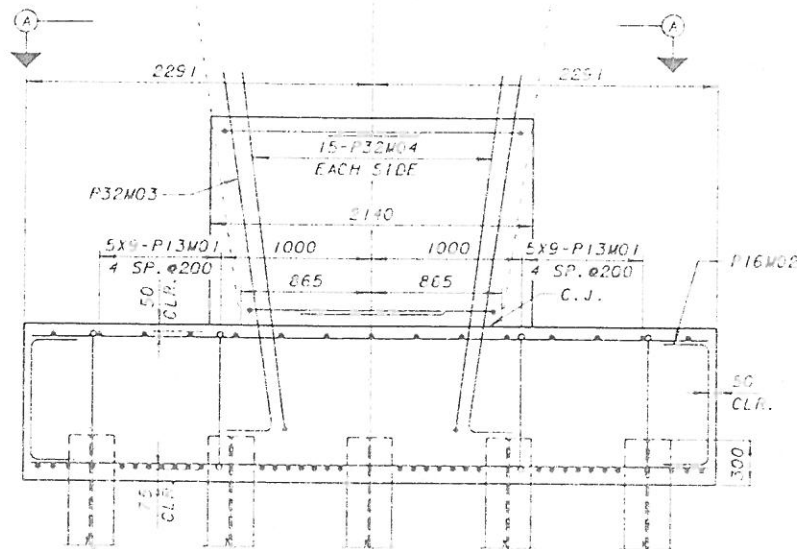
* THE SIZE OF LOWER PORTION OF THE COLUMNS IS 550 x 2140 mm. THE 480 mm & 1730 mm DIMENSIONS ARE GIVEN TO PROPERLY LOCATE THE DOVELS IN THE FOOTING.

PIER 1 -
FORT WASHINGTON WAY

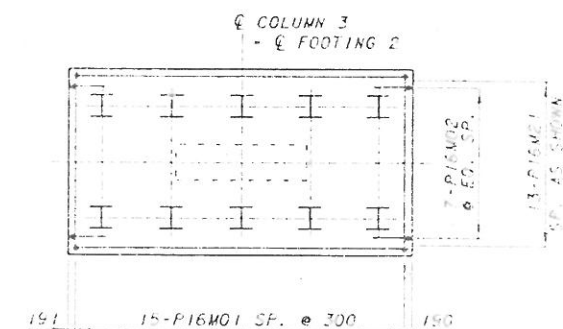
COLUMN 3



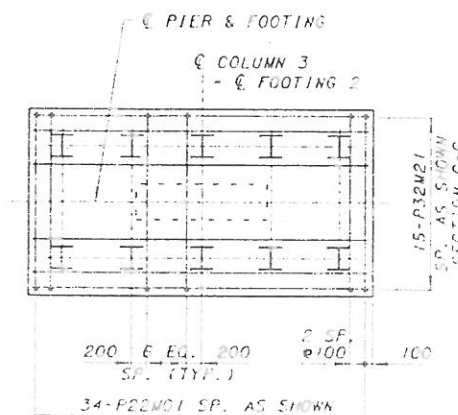
SECTION C-C



SECTION B-B
SEE SHEET 9/19 FOR SECTION A-A



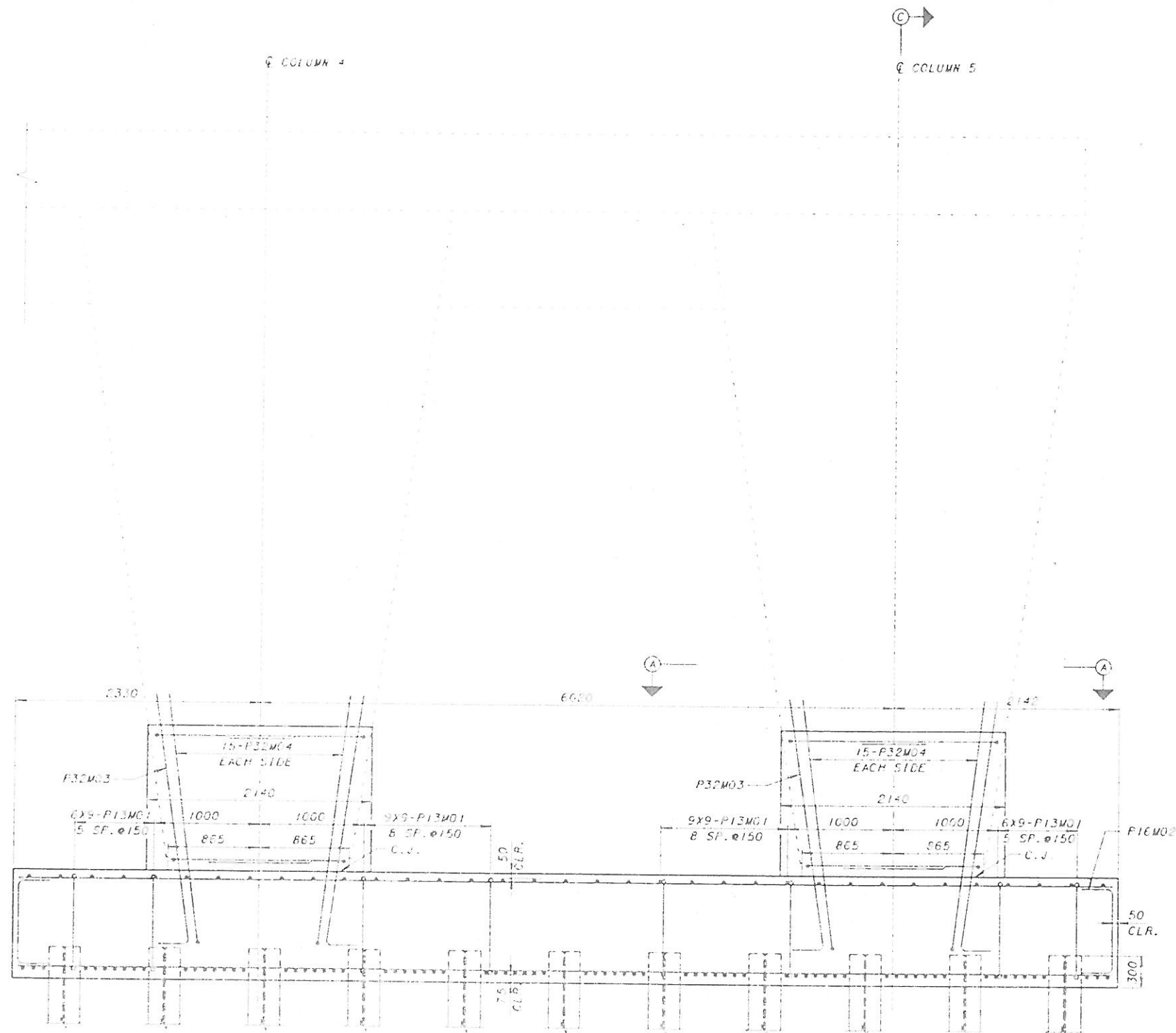
PLAN FOOTING 2 TOP REINFORCING



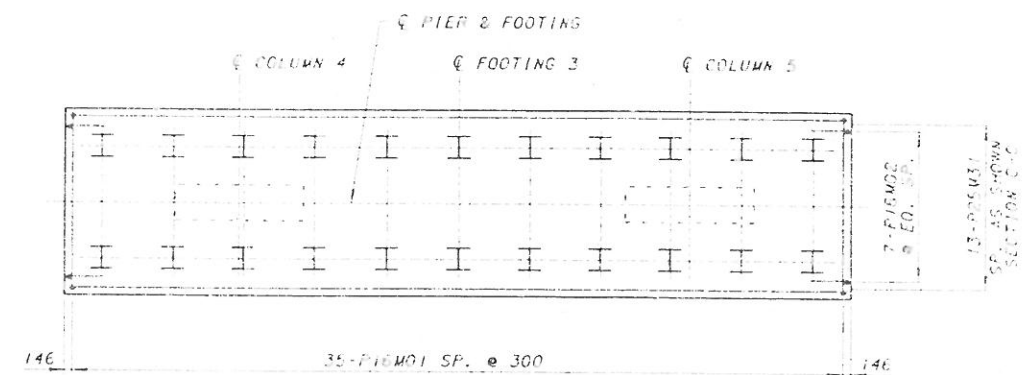
PLAN FOOTING 2 BOTTOM REINFORCING

1412/01 6-1 JUN 12 09:33:15 2003

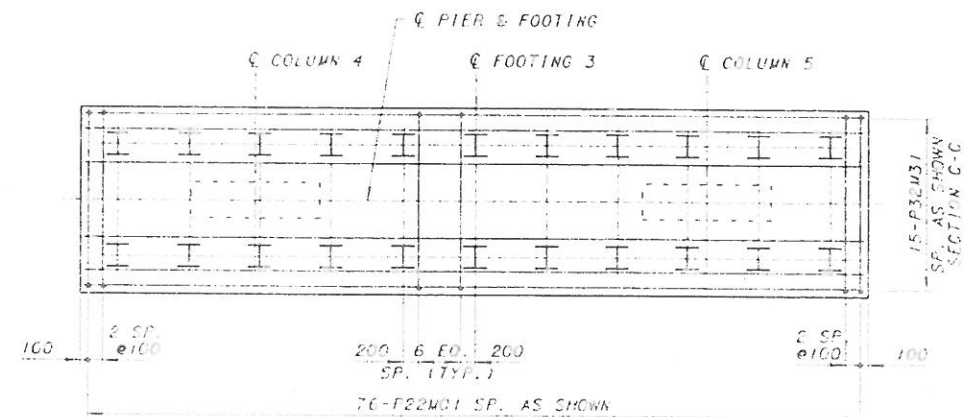
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SECTION B-B
SEE SHEET 10/19 FOR LOCATION OF SECTION B-B
SEE SHEET 9/19 FOR SECTION A-A



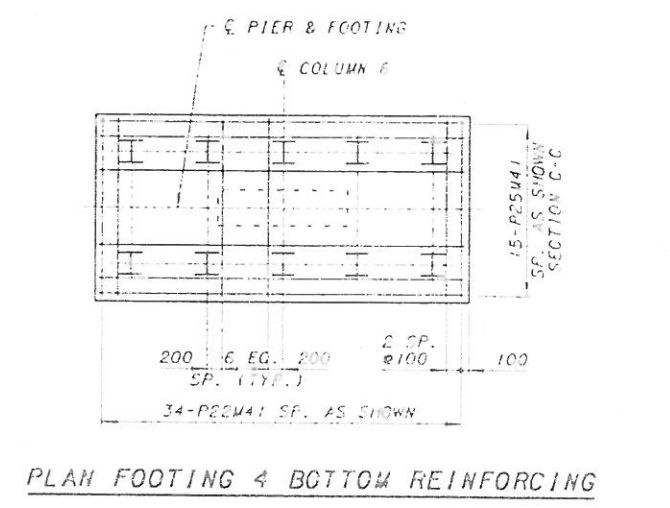
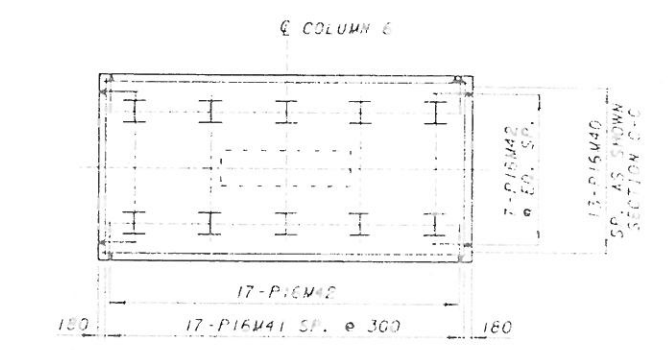
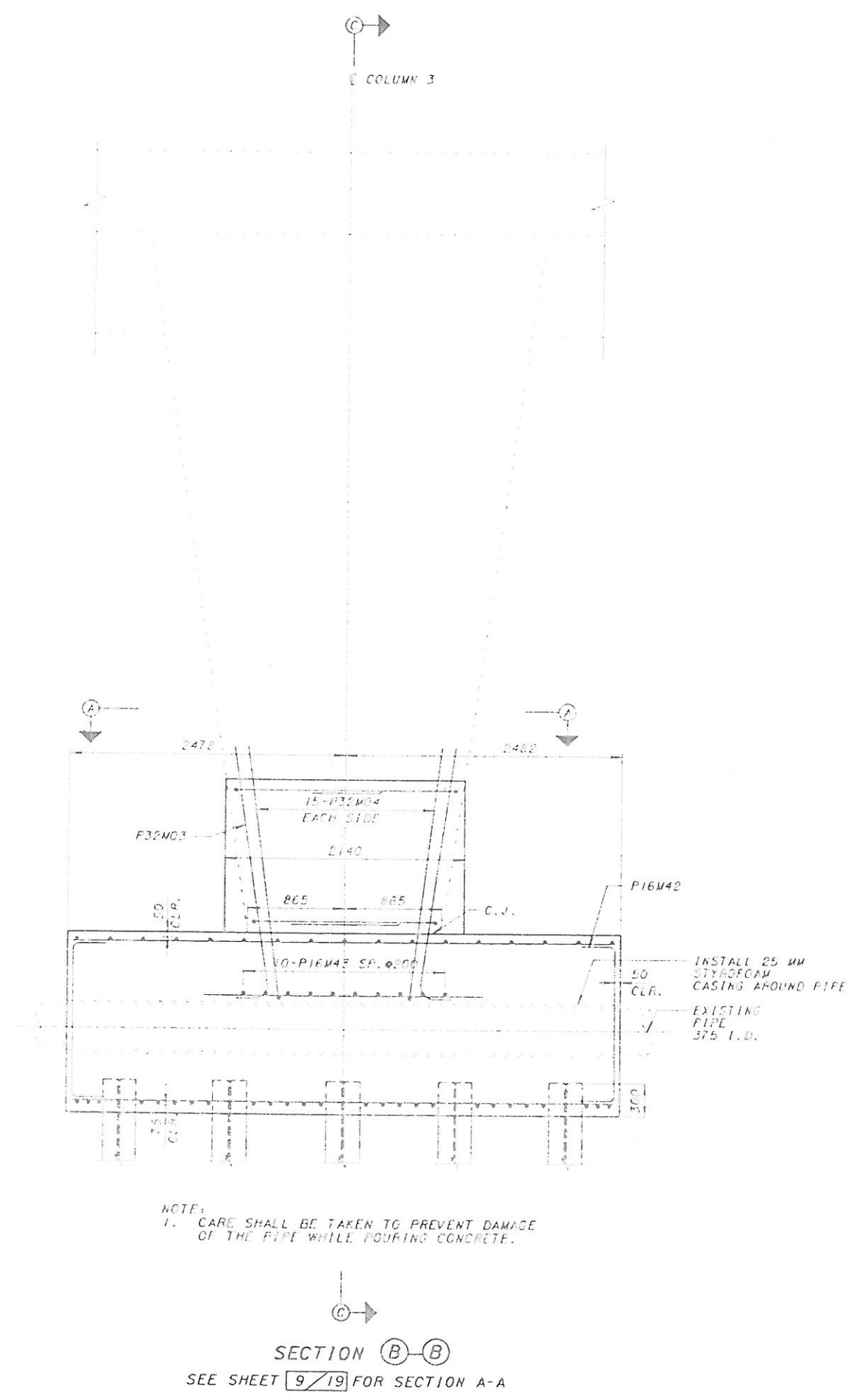
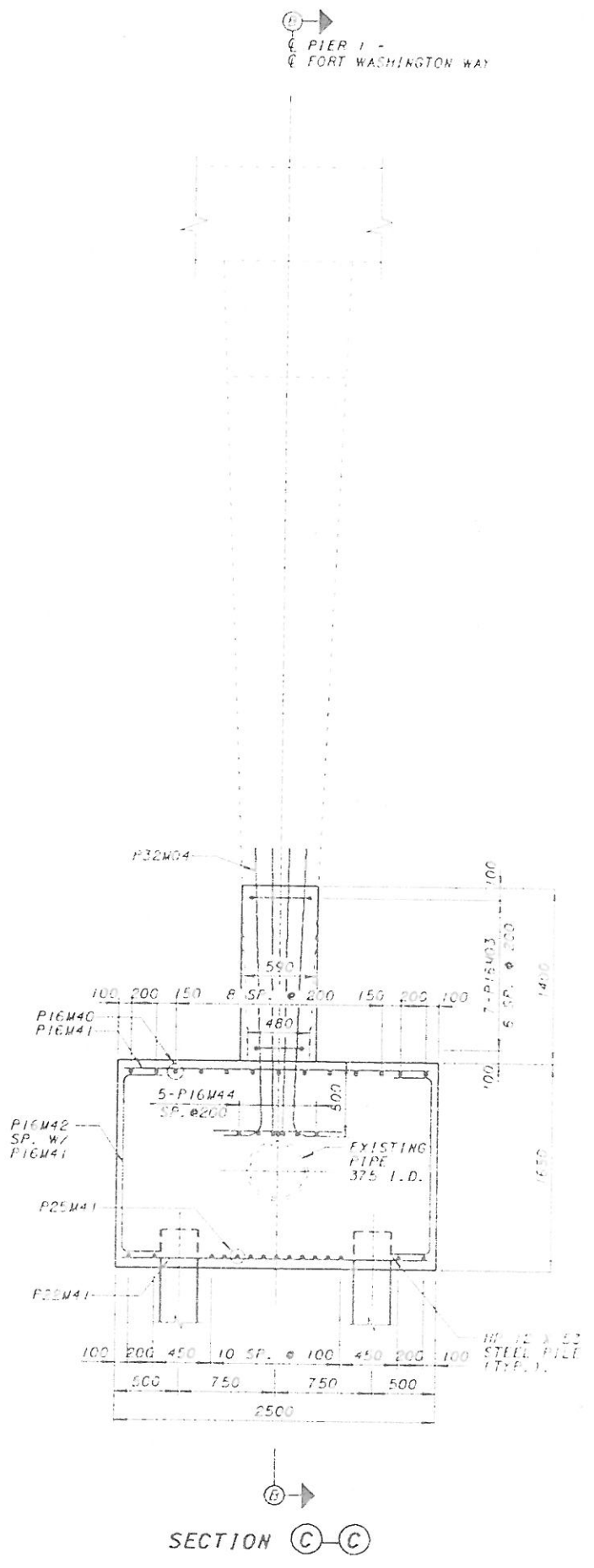
PLAN FOOTING 3 TOP REINFORCING

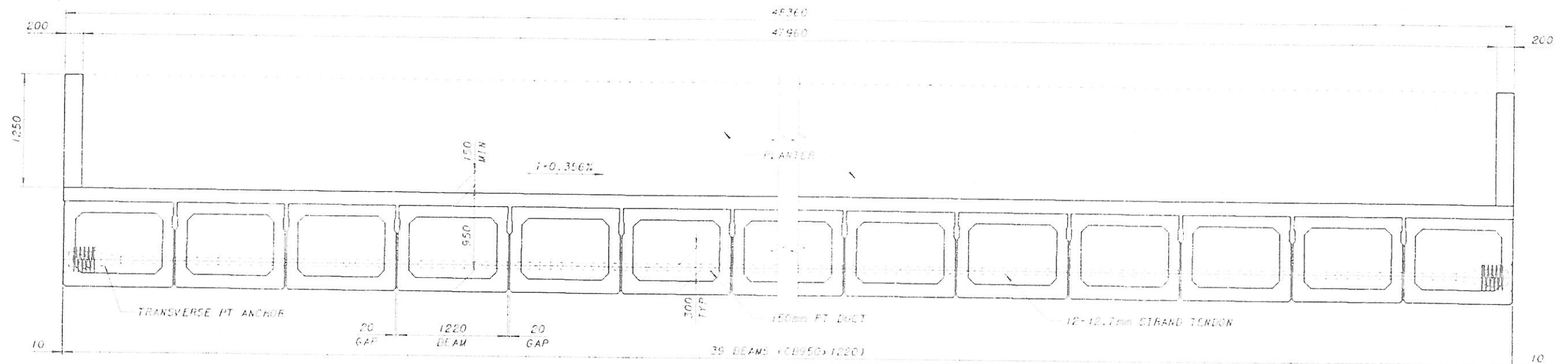


PLAN FOOTING 3 BOTTOM REINFORCING

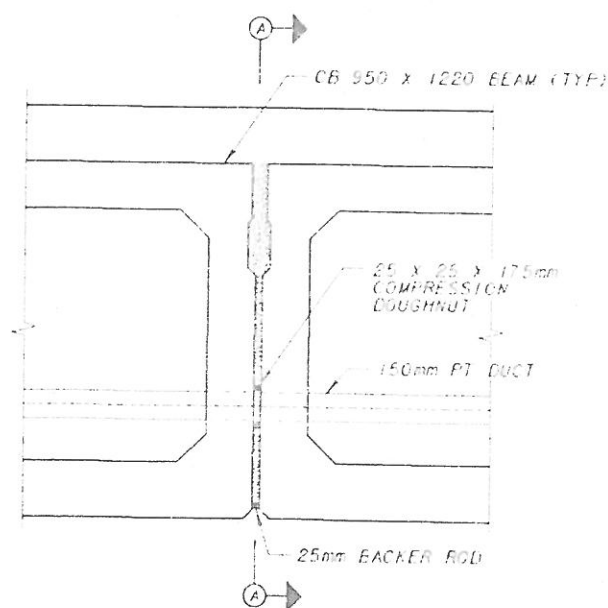
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10/12/01

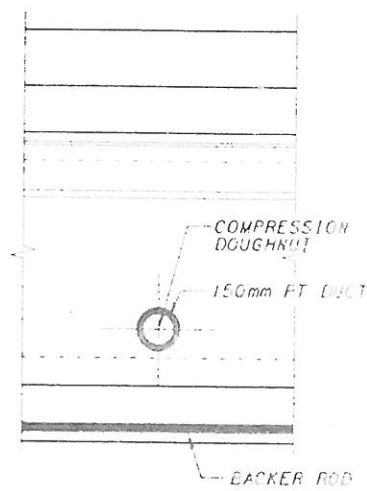




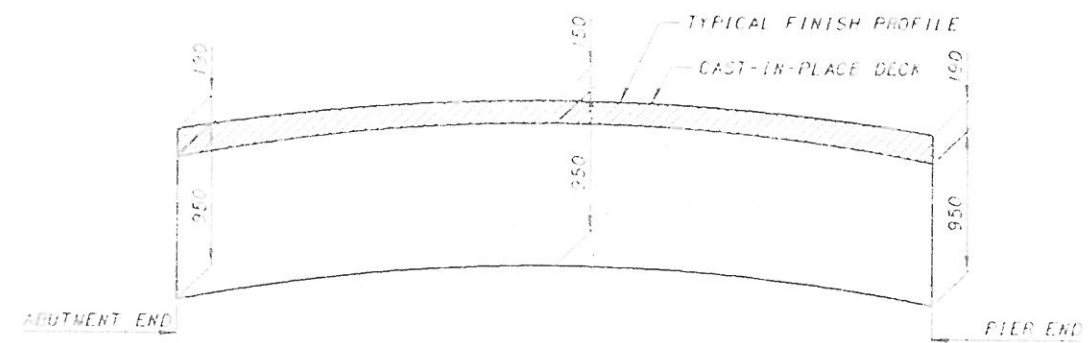
TYPICAL CROSS SECTION



BEAM DETAIL

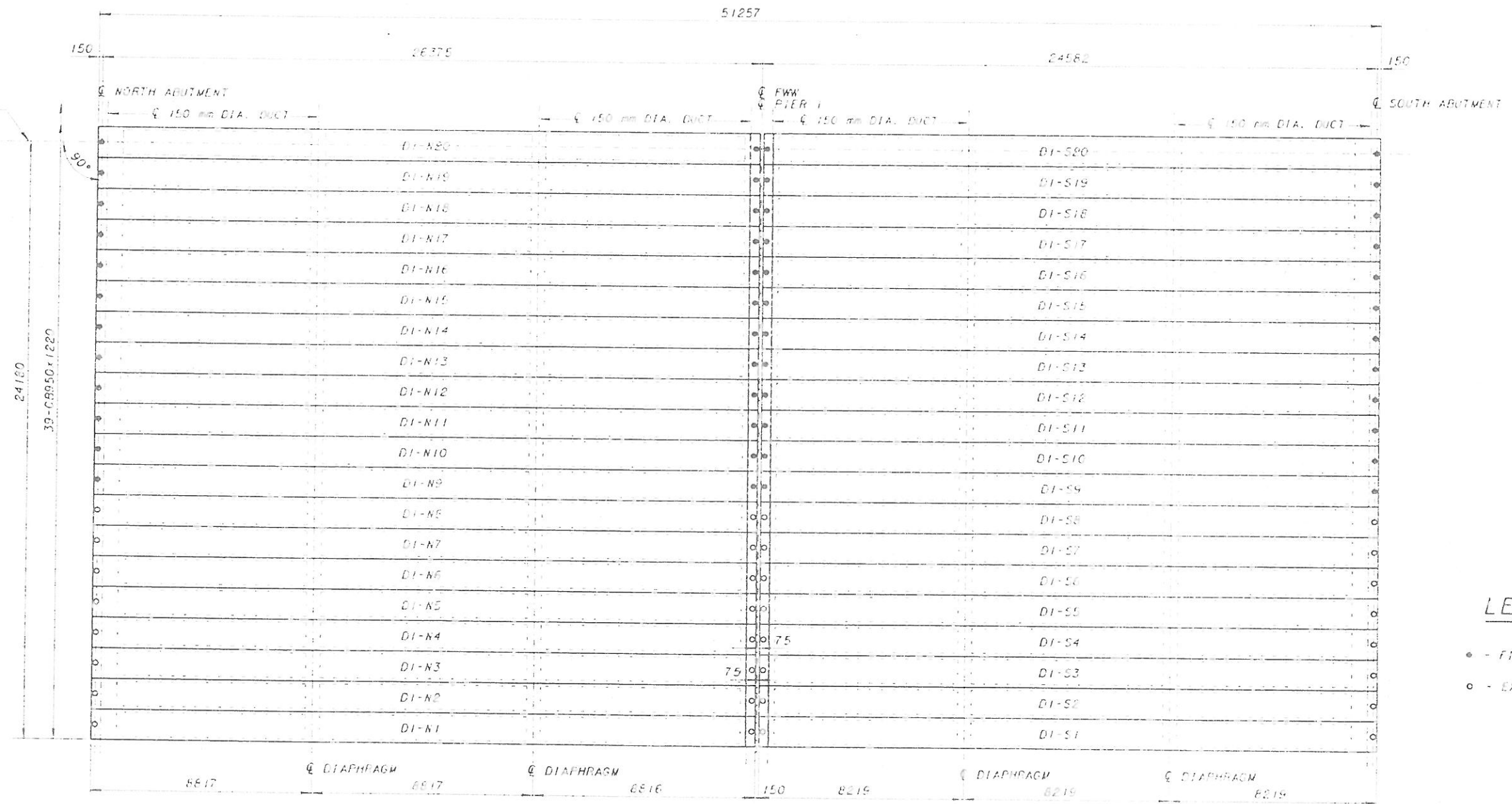


SECTION A-A



BEAM CAMBER DIAGRAM

NOTES:
1. SEE NOTES 1 AND 2 ON SHEET



LEGEND

- - FIXED ANCHOR DOWELS
- - EXPANSION ANCHOR DOWELS, U.N.O.

FRAMING PLAN

- NOTES:
1. MINIMUM CONCRETE STRENGTH OF BEAMS AFTER 28 DAYS: 58.7 MPa, MINIMUM CONCRETE STRENGTH AT RELEASE: 37.2 MPa.
 2. PRESTRESSING STRAND TO BE UNCOATED, LOW RELAXATION 12.7 mm DIA., SEVEN WIRES (AS-108 80, mm), ASTM A416, GRADE 270, 18-1800 MPa, INITIAL STRESS = 0.75 f_s = 1400 MPa.
 3. REINFORCING STEEL TO BE ASTM A615M OR A617M GRADE 400.
 4. TOP OF BEAMS ARE TO BE SCORED TRANSVERSELY AT ABOUT 75 mm CENTERS WITH A POINTED TOOL (MAX. DEPTH OF SCORING TO BE 6 mm).
 5. OMIT KEYWAY ON EXTERIOR BEAMS.
 6. SEE NOTES 1 AND 2 ON SHEET 2/13

DESIGN AGENCY
PARSONS BRINCKERHOFF, INC.
518 ELM STREET, SUITE 2500
CINCINNATI, OH 45202-2700
2025

DATE
6-30-00

REVISED
STRUCTURE FILE NUMBER

DRAWN
SRL

REVIEWED
JSH

FILED
VR

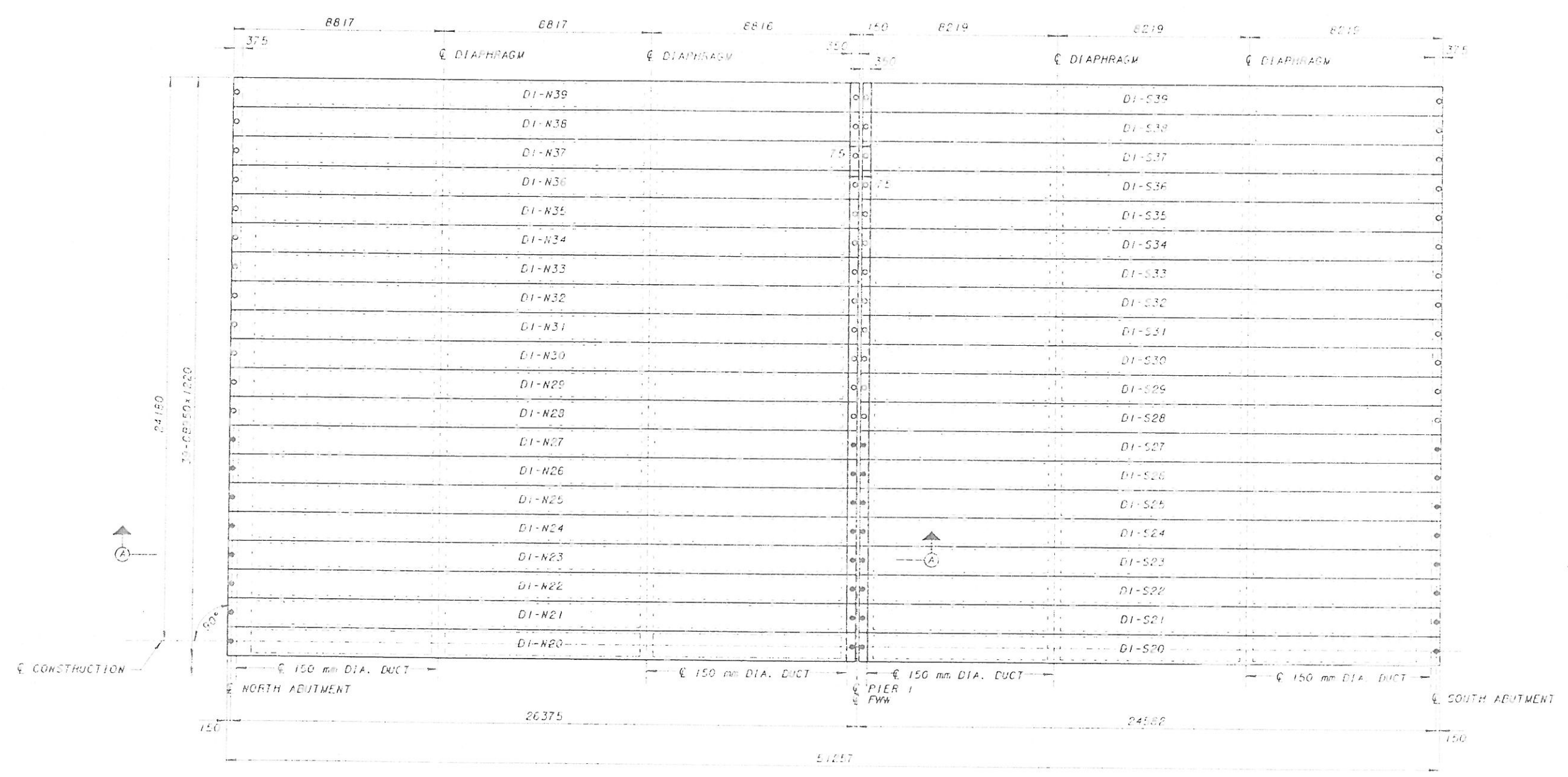
CHECKED
JSH

HAMILTON COUNTY
STA. 100+427.629
STA. 100+477.000

FRAMING PLAN (1 OF 2)
DECK NUMBER 1
OVER FORT WASHINGTON WAY

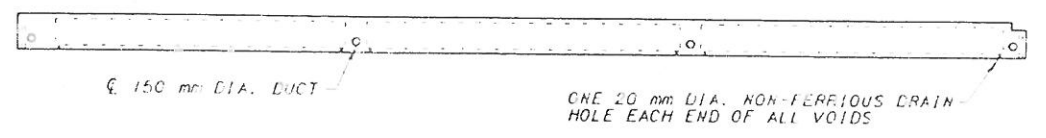
CITY OF CINCINNATI
CONTRACT NO. 75X5753

15/19



FRAMING PLAN

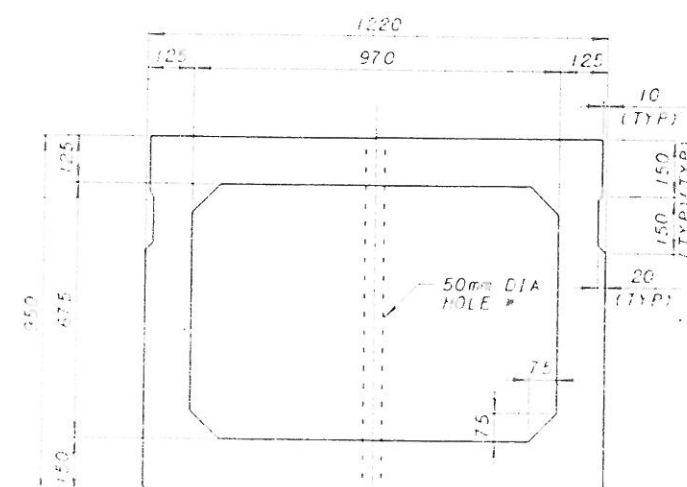
NOTES:
1. WORK TOGETHER WITH FRAMING PLAN SHEET 15/19



SECTION A-A

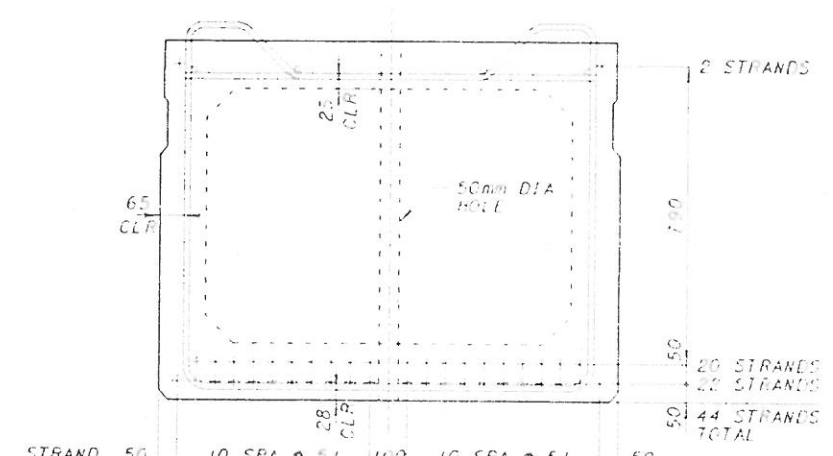
FILED IN 10-07-2019 10:00 AM
10/27/19
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 DESIGN AGENCY PARSONS BRINCKERHOFF, INC. 312 ELM STREET, SUITE 2500 CINCINNATI, OH 45202-2720 200 100%	DATE 6-30-00	REVISION STRUCTURE FILE NUMBER
	DRAWN SRL	CHECKED JSH
HAMILTON COUNTY STA. 100+427.529 STA. 100+477.000		
FRAMING PLAN (2 OF 2) DECK NUMBER 1 OVER FORT WASHINGTON WAY		
CITY OF CINCINNATI CONTRACT NO. 75X5753		
16/19		

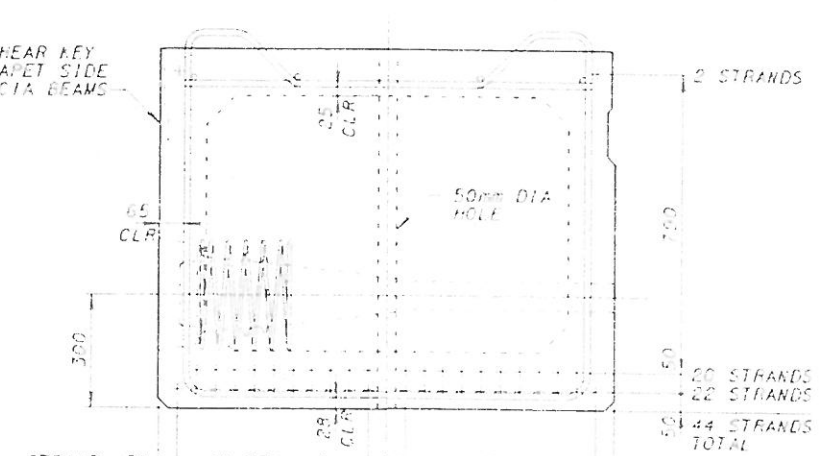


CROSS SECTION CB950x1220 BEAM

* FOR 25mm DIA SMOOTH DOWEL (L-1340)

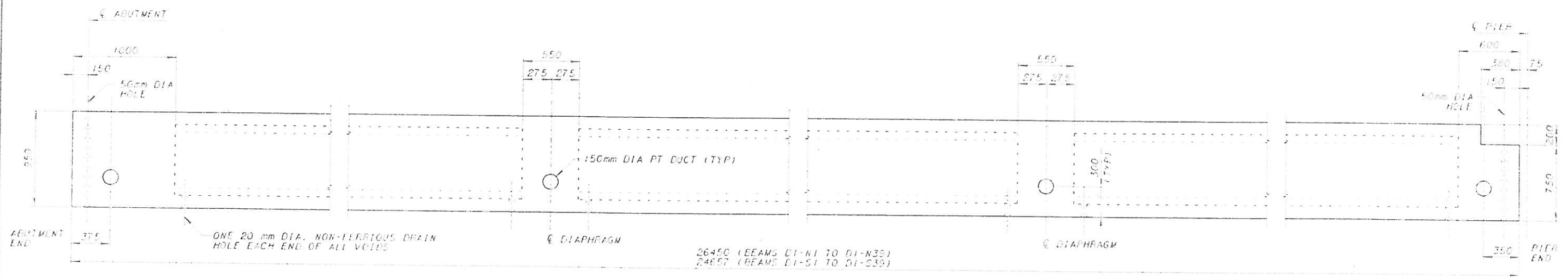


CB950x1220 TYPICAL BEAM

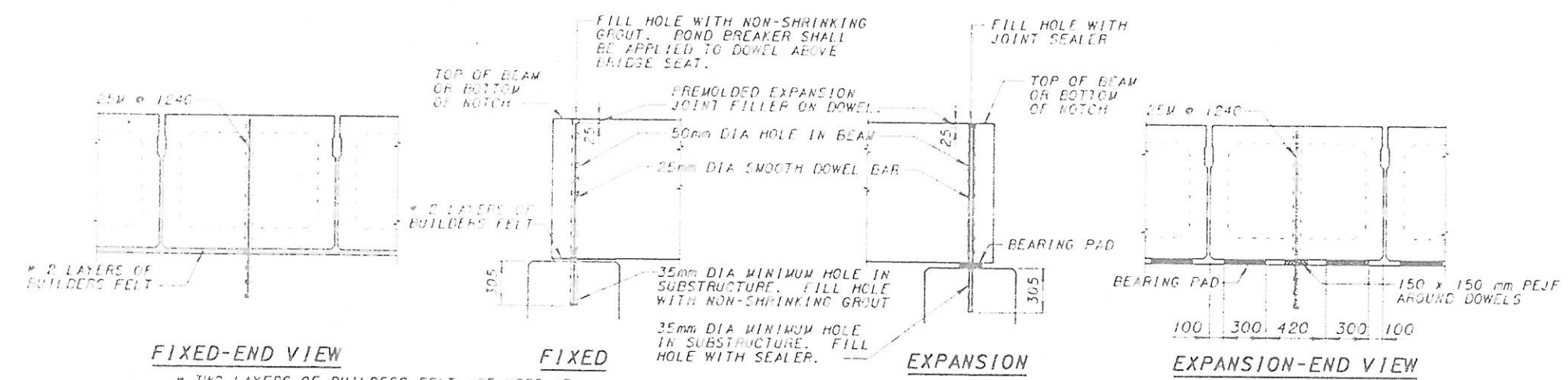


CB950x1220 END BEAM

OMIT SHEAR KEY ON PARAPET SIDE OF FASCIA BEAMS



ELEVATION



FIXED-END VIEW

FIXED

EXPANSION

EXPANSION-END VIEW

ANCHOR DOWELS

DOWEL HOLE LOCATION

* TWO LAYERS OF BUILDERS FELT ARE USED AT ABUTMENTS IN CONJUNCTION WITH FIXED ANCHOR DOWELS. BEARING PADS SHALL BE USED AT CENTRAL PIER IN CONJUNCTION WITH FIXED ANCHOR DOWELS.

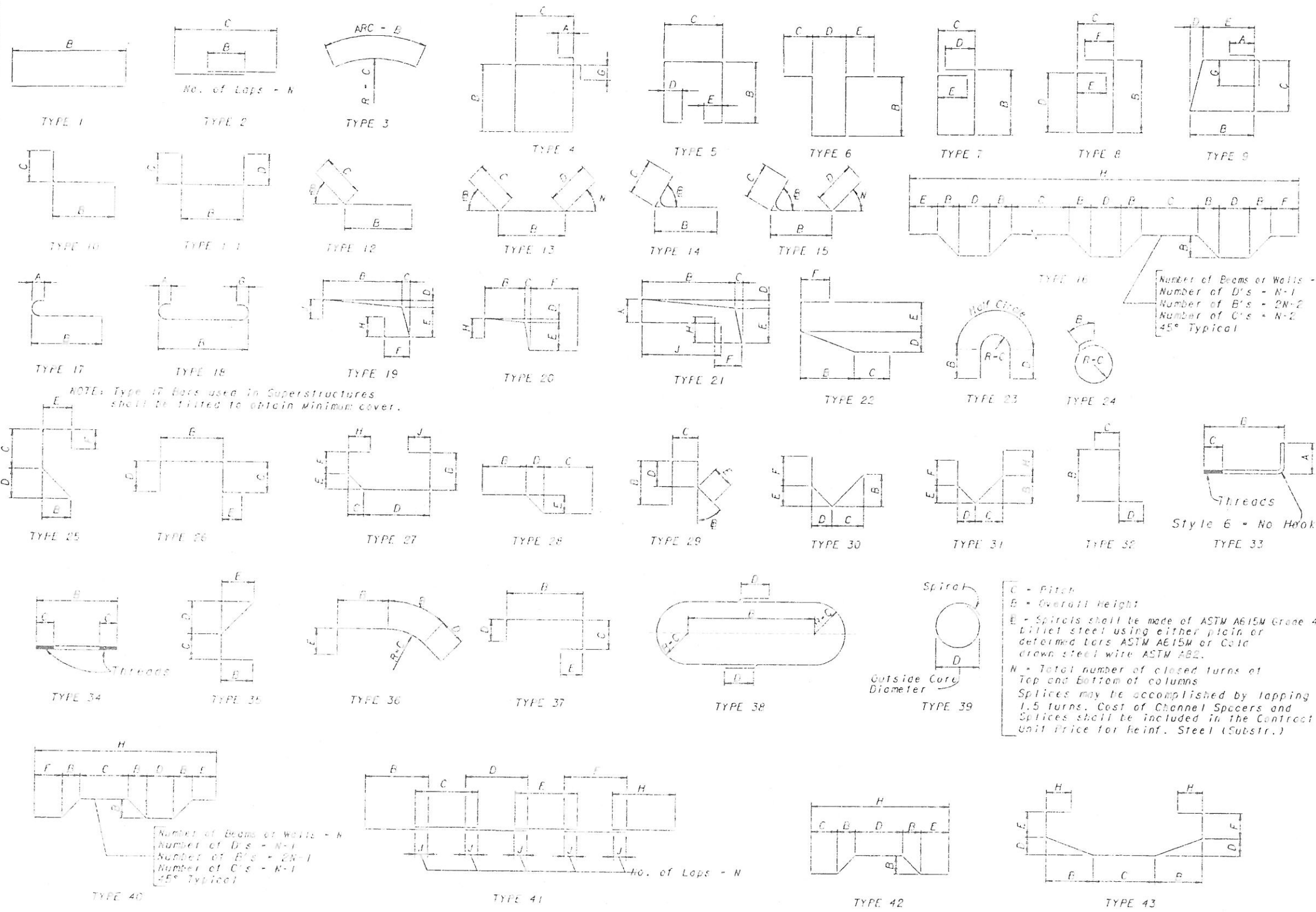
NOTE: DOWEL CLEARS ELASTOMERIC BEARING

NOTES:
1. SEE NOTES 1 AND 2 ON SHEET 2/19

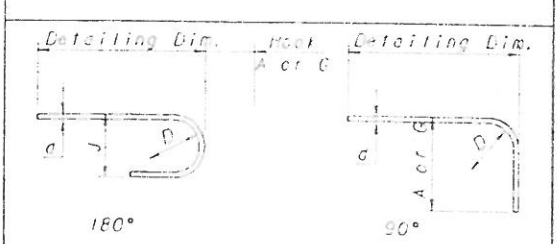
Rev. Jan. 12, 1940-1946, 1947

1/2" 10'

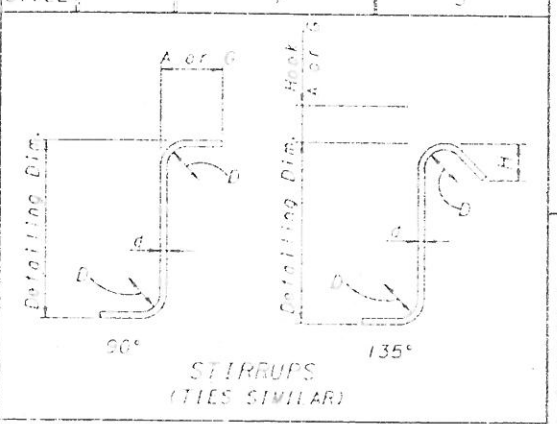
Reinforcement Fabrication and Construction Division



NOTE: For Bar Dimensions See REINFORCING BAR LIST Sheet.



BAR SIZE	D	180° HOOKS A OR G	90° HOOKS A OR G
#10	60	125	80
#13	80	150	105
#16	95	175	130
#19	115	200	155
#22	135	250	180
#25	155	275	205
#29	240	375	300
#30	275	425	335
#36	305	475	375
#43	465	675	550
#57	610	925	725



BAR SIZE	D	90° HOOKS A OR G	135° HOOKS A OR G	H*
#10	40	105	105	65
#13	50	115	115	80
#16	65	155	140	95
#19	115	305	205	115
#22	135	355	230	135
#25	155	410	270	155

* Dimension is approximate.
Hook Styles Detailed on this sheet are for Illustration Only.
Actual Hook Style for any particular bar will be shown under A or G heading on REINFORCING BAR LIST sheet.
All dimensions are in millimeters (mm), except as noted.
All dimensions are cut-to-out.

DESIGN AGENCY
HAMILTON COUNTY
STA. 100+427.629
STA. 100+477.000
STANDARD BAR BENDING DETAILS
DECK NUMBER 1
OVER FORT WASHINGTON WAY
CITY OF CINCINNATI
CONTRACT NO. 75X5753
16/19

1/12/01
1/12/01
1/12/01

1/12/01
1/12/01
1/12/01

MARK	TOTAL	LENGTH	WEIGHT	TYPE	STYLE		DIMENSIONS (mm)										NO	ANG	REMARKS
	NUMBER	(mm)	(kg)		A	G	B	C	D	E	F	H	J	K	N	O			
PIER FOOTING																			
P13M01	900	1005	899	10	4	5	775												
P16M01	126	3750	733	11			2400	675	675										
P16M02	56	1485	129	11			885	300	300										
P16M40	13	4860	98	1			4860												
P16M41	17	2400	63	1			2400												
P16M42	48	2085	155	11			1485	300	300										
P16M43	10	1000	16	1			1000												
P16M44	5	2500	19	1			2500												
P22M01	255	3750	3800	11			2400	750	750										
P22M41	34	2400	324	1			2400												
P25M11	13	9900	511	1			9900												
P25M21	13	4480	231	1			4480												
P25M31	13	10390	537	1			10390												
P25M41	15	4860	290	1			4860												
P25M51	13	14000	723	41			6700	7300					1200		1				
P32M11	15	9900	951	1			9900												
P32M21	15	4480	430	1			4480												
P32M31	15	10390	508	1			10390												
P32M51	15	14800	1422	41			7100	7700					2000		1				
COLUMN STUB																			
P16M03	18	3635		11			485	1570	1570										
	SETS OF	70	700				70												
P16M03	7	3525		11			385	1570	1570										
P16M04	27	790		10	4	5	495												
	SETS OF	70	216				70												
P16M04	7	680		10	4	5	385												
P32M03	72	3050	1406	12			2750	300								98			
P32M04	270	3050	5214	12			2750	300								92			
NORTH ABUTMENT																			
A16M02	392	2450	1491	11			820	815	815										
A16M03	4	2085	13	11			785	650	650										
A16M04	66	3420	350	4	5	5	785	785											
A16M01	4	50530	314	2			400	48930								4			
A19M01	8	52530	939	2			900	48930								4			
SOUTH ABUTMENT																			
A16M02	332	2450	1262	11			820	815	815										
A16M03	16	2085	52	11			785	650	650										
A16M04	66	3420	361	4	5	5	785	785											
A16M11	12	13600	253	2			400	13200								1			
A16M21	4	4960	31	1			4960												
A19M11	24	14100	756	2			900	13200								1			
A19M21	8	4960	89	1			4960												

NOTES:

1. ALL DIMENSIONS ARE OUT TO OUT OF BAR EXCEPT ON STANDARD 90°, 135° AND 180° HOOKS.
2. DIMENSIONS ON HOOKS TO BE SHOWN ONLY WHERE NECESSARY TO RESTRICT HOOK SIZE, OTHERWISE STANDARD HOOKS ARE TO BE USED.
3. ALL BENDS SHOWN ARE BENT AROUND A STANDARD MANDREL. EXCEPT WHERE RADIUS OR DIAMETER IS INDICATED.
4. ALL BARS ARE EPOXY COATED.
5. SP BARS ARE SPIRAL BARS. THE WEIGHT OF THE SPIRAL INCLUDES 11 KILOGRAMS FOR 3 ADDITIONAL COILS, 1½ COILS AT EACH END PER AASHTO 8.18.2.2.4 THE WEIGHT OF THE SPIRAL INCLUDES 13.6 KILOGRAMS FOR 3 ADDITIONAL COILS, 1½ COILS AT EACH END PER AASHTO 8.18.2.2.4. CONCRETE SPACERS OR OTHER APPROVED NON-CORROSIVE SPACING DEVICES SHALL BE USED AT SUFFICIENT INTERVALS (NEAR THE BOTTOM AND AT INTERVALS NOT EXCEEDING 3.050m) TO INSURE CONCENTRIC SPACING FOR THE ENTIRE CASE LENGTH. SPACERS SHALL BE CONSTRUCTED OF APPROVED MATERIAL EQUAL IN QUALITY AND DURABILITY TO THE CONCRETE SPECIFIED FOR THE COLUMN. THE SPACERS SHALL HAVE ADEQUATE DIMENSIONS TO ENSURE A MINIMUM 75mm CLEAR SPACE BETWEEN THE OUTSIDE OF THE REINFORCING CAGE AND THE DESIGN DIMENSION OF THE COLUMN. CYLINDRICAL CONCRETE FEET (BOTTOM SUPPORTS) SHALL BE PROVIDED TO ENSURE THAT THE BOTTOM OF THE CAGE IS MAINTAINED AT THE PROPER DISTANCE ABOVE THE BASE.
6. THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST TWO DIGITS INDICATE THE BAR SIZE NUMBER. FOR EXAMPLE, P19M01 IS A NO. 19M BAR.
7. FOR STYLE OF HOOK "A" OR "G" AND HOOK DIMENSIONS REFER TO THE STANDARD BAR BENDING DETAILS. TOTAL LENGTH AND WEIGHT OF BARS INCLUDE HOOKS.