

FORT

WASHINGTON

WAY

2000

CITY OF CINCINNATI

DEPARTMENT OF PUBLIC WORKS

DIVISION OF ENGINEERING

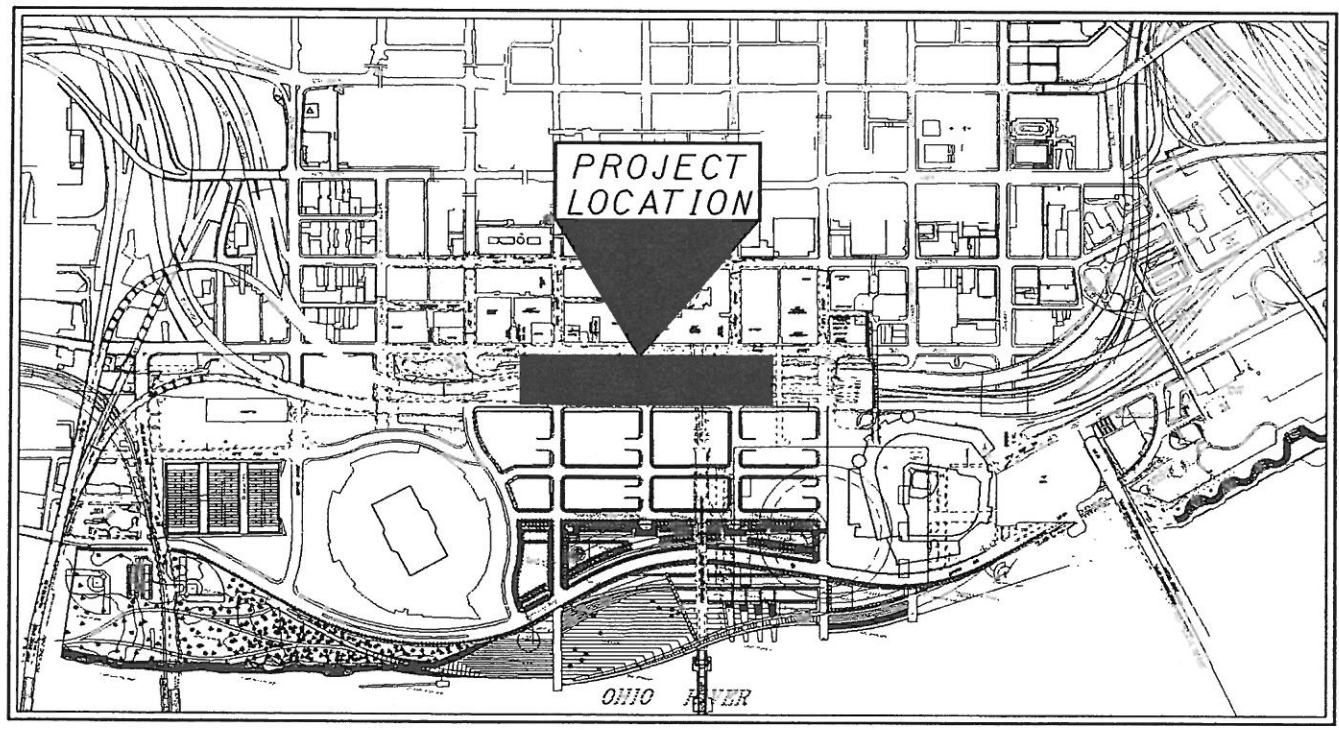
PROPOSED IMPROVEMENT

FORT WASHINGTON WAY

RECONSTRUCTION PROGRAM

CONTRACT 26

DECK 3



LOCATION MAP

Plans Reviewed By:

PB

100 YEARS

Plans Prepared For:

CITY OF CINCINNATI

OHIO

City of Cincinnati

Engineering Division

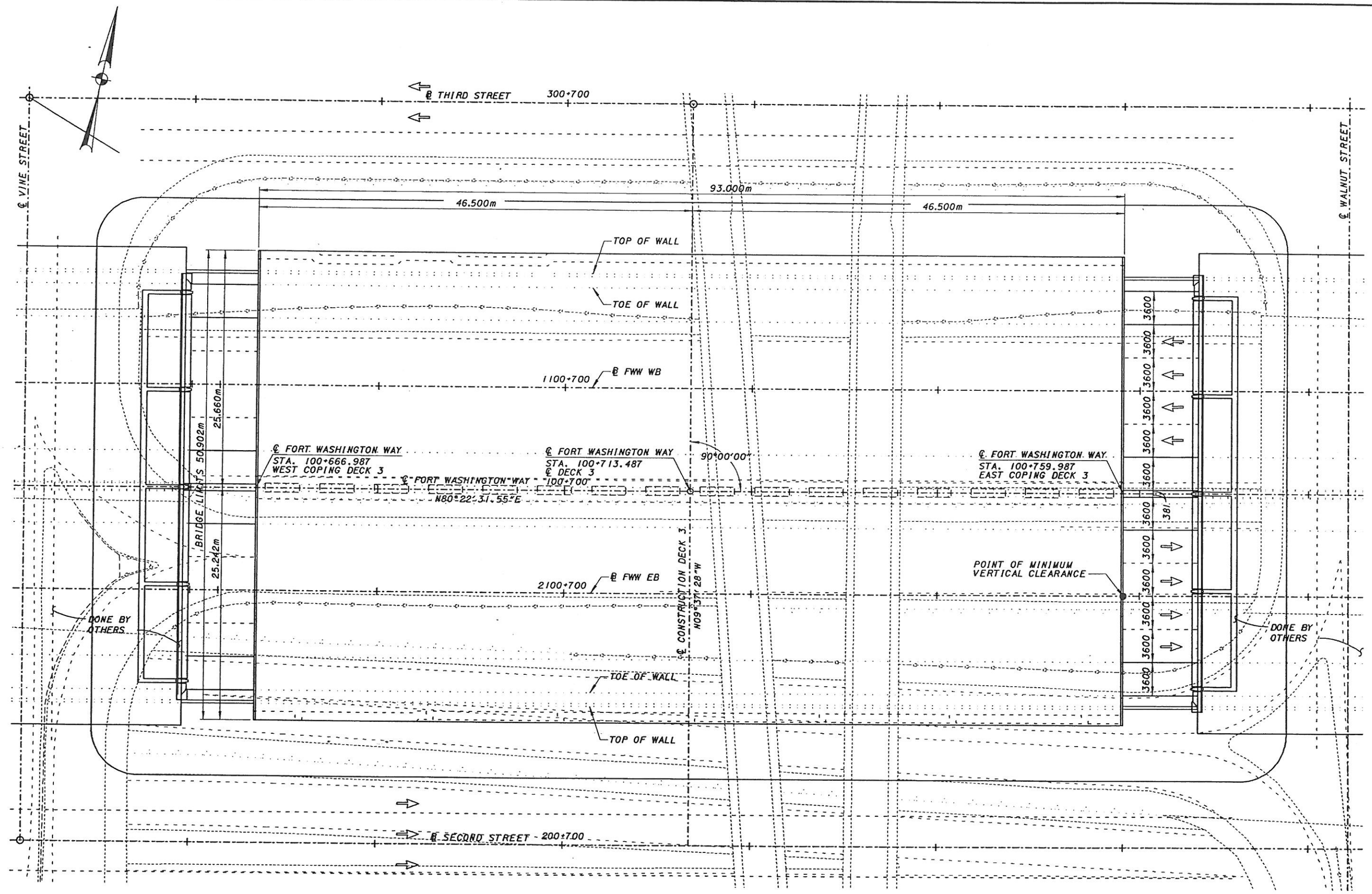
Department of Public Works

CITY OF CINCINNATI CONTRACT No. 75X5753	RAILROAD INVOLVEMENT		CONSTRUCTION PROJECT NO.	PID NO.	FEDERAL PROJECT NO.

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1/12/01

Fri Jan 12 10:04:18 2001



PLAN

	CITY OF CINCINNATI CONTRACT NO. 75X5753	SITE PLAN (SHEET 1 OF 2) DECK NUMBER 3 OVER FORT WASHINGTON WAY				HAMILTON COUNTY STA. 100+666.987 STA. 100+759.987	PREPARED	DRAWN	REVISED	DATE	DESIGN AGENCY
		SRL	SRL	FILE NUMBER	6-30-00		PARSONS BRINCKERHOFF, INC. 312 ELM STREET, SUITE 2500 CINCINNATI, OH 45202-8720				

GENERAL NOTES

STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS

THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS APPLY TO THIS PROJECT:
SUPPLEMENTAL SPECIFICATION 844 : HIGH PERFORMANCE CONCRETE FOR STRUCTURES
SUPPLEMENTAL SPECIFICATION 515 : POST-TENSIONING CONCRETE
SUPPLEMENTAL SPECIFICATION : MASS CONCRETE

DESIGN SPECIFICATIONS

THIS PROJECT IS DESIGNED IN METRIC SI UNITS, ALL DIMENSIONS ARE IN MILLIMETERS (mm), AND ALL ELEVATIONS ARE IN METERS (m), UNLESS NOTED OTHERWISE.

THIS STRUCTURE CONFORMS TO

"STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, SIXTEENTH EDITION, 1996, INCLUDING THE 1997 INTERIM SPECIFICATIONS

STATE OF OHIO "BRIDGE DESIGN MANUAL"

STATE OF OHIO "CONSTRUCTION AND MATERIAL SPECIFICATIONS", 1997.

DESIGN LOADINGS

DEAD LOADS

UNIT WEIGHT OF SOIL IN PLANTERS - 19kN/m³
MAXIMUM HEIGHT OF SOIL - 1.22 METER

UNIT WEIGHT OF REINFORCED CONCRETE - 23.6 kN/m³

WEIGHT OF THE PRESTRESSED CONCRETE BOX BEAMS INCLUDING 150 mm DECK - 13.67 kN/m²

ADDITIONAL SUPERIMPOSED DEAD LOAD - 1.8 kN/m²

SNOW LOAD

SNOW LOADING - 0.72 kN/m²

THERMAL FORCES

ANTICIPATED THERMAL MOVEMENTS AND FORCES WERE CALCULATED BASED ON THE FOLLOWING:

DESIGN MEAN TEMPERATURE = 21 DEG. C (68 DEG. F.)
TEMPERATURE RISE = 19 DEG. C (35 DEG. F.)
TEMPERATURE FALL = 25 DEG. C (45 DEG. F.)
COEFFICIENT OF LINEAR EXPANSION = 0.0000108 PER DEGREE C.

SEISMIC FORCES

SITE HORIZONTAL ACCELERATION = 0.06g. DESIGN DETAILS IN ACCORDANCE WITH AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, SIXTEENTH EDITION, SEISMIC PERFORMANCE CATEGORY A AND ODOT BDM, 3.1.2.1.

LOADING COMBINATIONS

LOAD FACTORS, ALLOWABLE OVERSTRESS AND LOADING COMBINATIONS ARE IN ACCORDANCE WITH AASHTO TABLE 3.22.1A.

CLEARANCE

A MINIMUM VERTICAL CLEARANCE BETWEEN THE RIDING SURFACE OF ROADWAY BELOW AND THE LOWEST POINT OF THE STRUCTURE ABOVE, IF IT ENCROACHES ONTO TRAFFIC LANES OF ROADWAY BELOW, SHALL BE 4.9 m FOR MAINLINE ROADWAYS, RAMPS AND LOCAL STREETS.

CONCRETE

MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'_c) SHALL BE AS FOLLOWS:
HIGH PERFORMANCE CONCRETE - COMPRESSIVE STRENGTH 38.0 MPa TO BE USED FOR PIER COLUMNS.

CONCRETE CLASS "C" - COMPRESSIVE STRENGTH 27.0 MPa TO BE USED FOR FOUNDATIONS AND ABUTMENTS.

CONSTRUCTION JOINTS IDENTIFIED ON THE DRAWINGS AS "ROUGHENED", SHALL BE CLEAN AND SHALL BE ROUGHENED TO A FULL AMPLITUDE OF APPROX. 6 mm.

REINFORCING STEEL

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615M, A616M OR A617M, GRADE 420 WITH MINIMUM YIELD STRENGTH OF 420 MPa.

ALL REINFORCING STEEL SHALL BE EPOXY COATED.

MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE SHOWN ON THE DRAWINGS. CONCRETE COVERS SHOWN ON THE DRAWINGS DO NOT INCLUDE PLACEMENT AND FABRICATION TOLERANCES, UNLESS SHOWN AS A "MINIMUM COVER".

DESIGN METHOD

POST-TENSIONED CONCRETE MEMBER SHALL BE DESIGNED IN ACCORDANCE WITH AASHTO 9.13.1.2.

REINFORCING CONCRETE MEMBERS SHALL BE DESIGNED USING THE STRENGTH DESIGN METHOD.

PROTECTION OF TRAFFIC DURING DEMOLITION

PRIOR TO THE DEMOLITION OF THE EXISTING STRUCTURE, THE CONTRACTOR SHALL SUBMIT HIS PLANS FOR THE PROTECTION OF TRAFFIC.

END BEARING PILES DRIVEN TO BEDROCK

THE ABUTMENT AND PIER PILES SHALL BE DRIVEN INTO BEDROCK A MINIMUM OF 300 mm OR TO PRACTICAL REFUSAL ON BEDROCK. REFUSAL SHALL BE CONSIDERED AS OBTAINED BY PENETRATING SOFT BEDROCK FOR SEVERAL MILLIMETERS AND HAVING A RESISTANCE OF 20 BLOWS PER 25 mm OR REFUSAL SHALL BE CONSIDERED AS OBTAINED AFTER THE PILE HAS PENETRATED BEDROCK AND THE PILE HAS THEN RECEIVED AT LEAST 20 BLOWS.

PILE LOADS - THE DESIGN ALLOWABLE CAPACITY FOR HP 12x53 PILES IS 980 kN IN COMPRESSION.

PILES NOT BEARING ON BEDROCK WILL NOT BE ACCEPTED. AUGER SOUNDINGS MAY BE REQUIRED IF THE PILES ENCOUNTER REFUSAL SIGNIFICANTLY ABOVE DESIGN ELEVATIONS.

PILE HAMMER

THE PILE HAMMER USED TO INSTALL THE PILES SHALL HAVE A STATE'S ENERGY RATING OF NOT LESS THAN 54000 JOULES. THIS REQUIREMENT DOES NOT RELIEVE THE CONTRACTOR FROM 108.05 WHICH STATES THAT THE CONTRACTOR IS TO PROVIDE SUFFICIENT EQUIPMENT FOR PROSECUTING THE REQUIRED WORK. REFER TO "ODOT'S MANUAL OF PROCEDURES FOR STRUCTURES" TO OBTAIN THE STATE'S ENERGY RATING.

STEEL POINTS

STEEL POINTS SHALL BE USED TO PROTECT THE TIPS OF THE PROPOSED "H" PILING. THE STEEL POINTS SHALL BE FURNISHED BY THE ASSOCIATED PILE AND FITTING CORPORATION, 262 RUTHERFORD BOULEVARD, CLIFTON, NEW JERSEY 07014; INTERNATIONAL CONSTRUCTION EQUIPMENT, INC., 301 WAREHOUSE DRIVE, MATHEWS, NORTH CAROLINA 28015; OR BY A MANUFACTURER THAT CAN FINISH A STEEL POINT THAT IS ACCEPTABLE TO THE ENGINEER.

TEST PILES

THE LOCATION OF THE TEST PILE SHALL BE DETERMINED BY THE ENGINEER. THE TEST PILE SHALL BE SUBJECTED TO DYNAMIC LOAD TESTING IN ACCORDANCE WITH CMS 523.

PRODUCTION PILES

AS DIRECTED BY THE ENGINEER, PRODUCTION PILES SHALL BE DYNAMICALLY TESTED IN ACCORDANCE WITH CMS 523. THE NUMBER OF PILES AND WHICH PILES TO BE TESTED WILL BE DETERMINED BY THE ENGINEER. THE DYNAMIC TESTING SHALL BE PERFORMED DURING BOTH THE INITIAL AND RESTRIKE DRIVING. THE DYNAMIC TESTING EQUIPMENT SHOULD CONFORM TO THE REQUIREMENTS OF ASTM D-4945-89, STANDARD TEST METHOD FOR HIGH STRAIN DYNAMIC TESTING OF PILES.

DESIGN AGENCY
PARSONS BRINCKERHOFF, INC.
512 ELM STREET, SUITE 8500
CINCINNATI, OH 45202-2720

DATE
6-30-00
REVISED
STRUCTURE FILE NUMBER

DRAWN
SRL
CHECKED
JSH
PREPARED
VR

LOCATION NAME
STA. 100+666.987
STA. 100+759.987

GENERAL NOTES
DECK NUMBER 3
OVER FORT WASHINGTON WAY

CITY OF CINCINNATI
CONTRACT NO. 75X5753

3/19

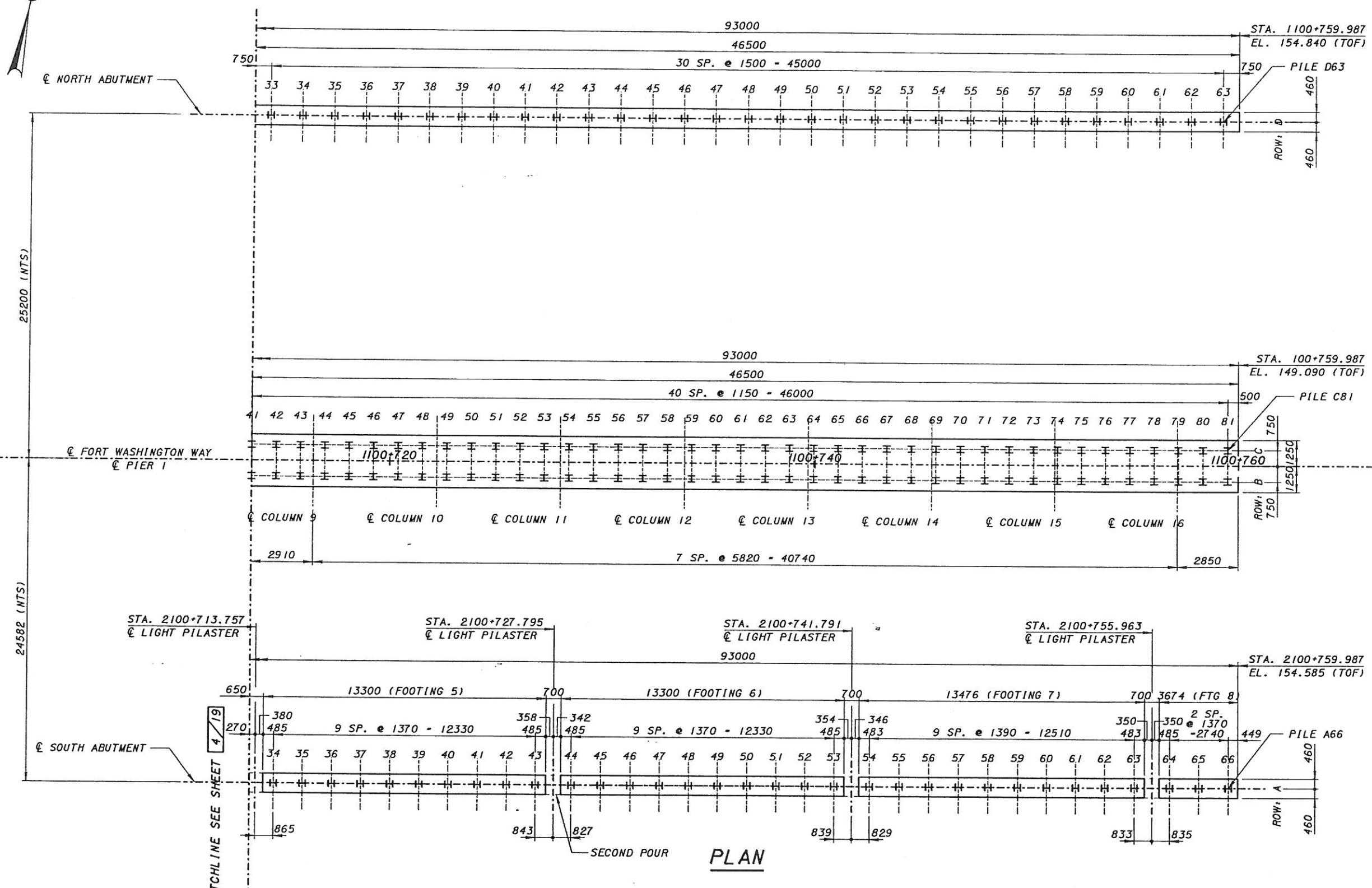
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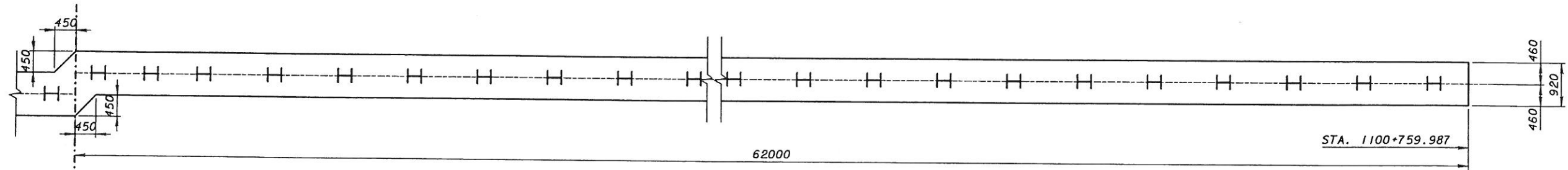
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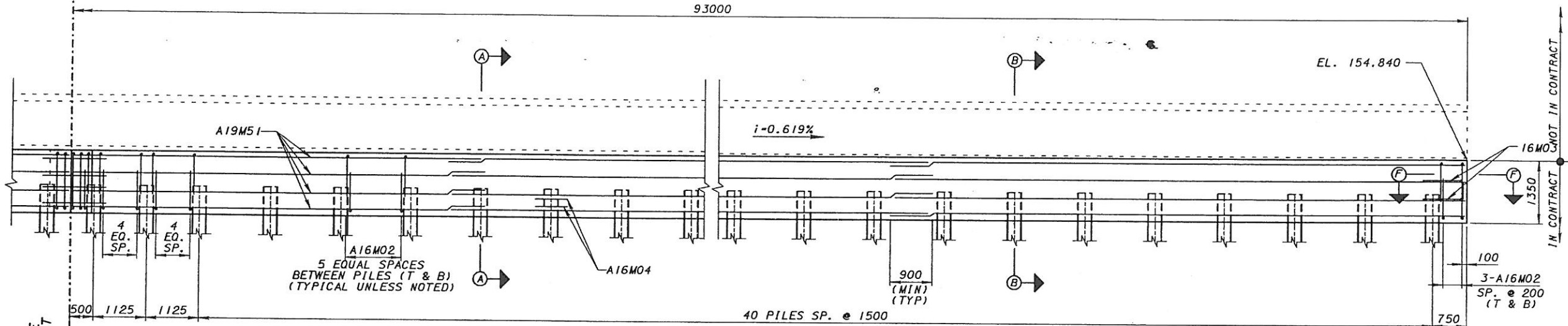




LEGEND
H-HP 12 X 53 STEEL PILE (TYP)
DRIVEN TO ROCK
980 kN PILE CAPACITY



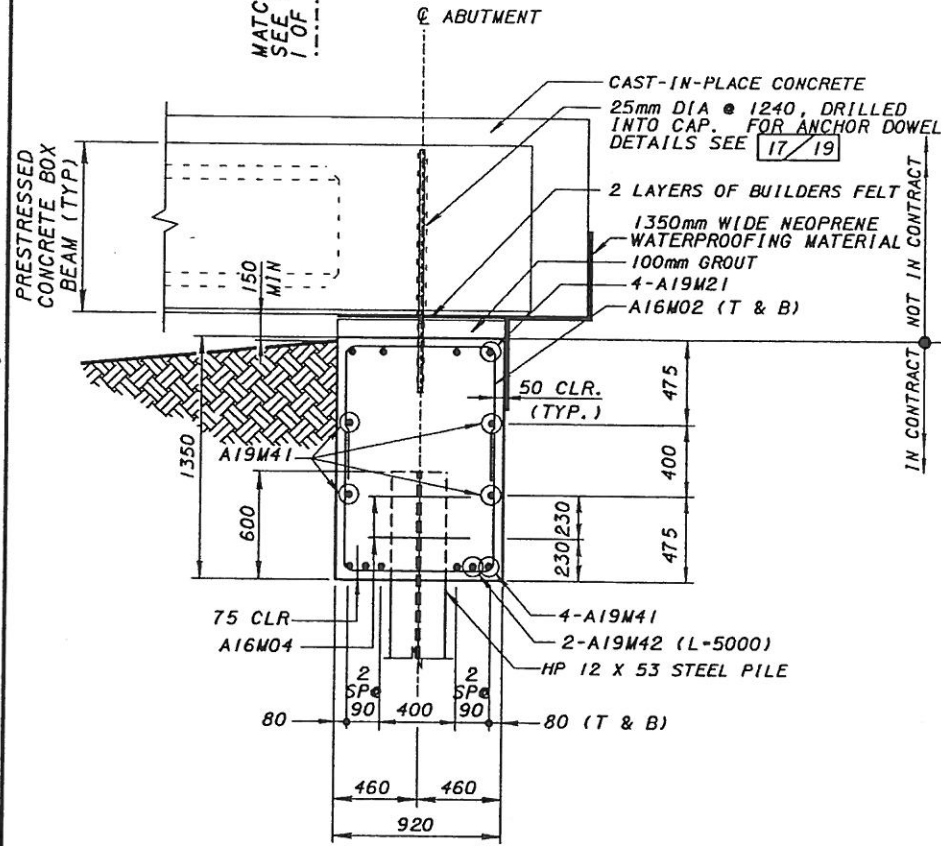
PLAN



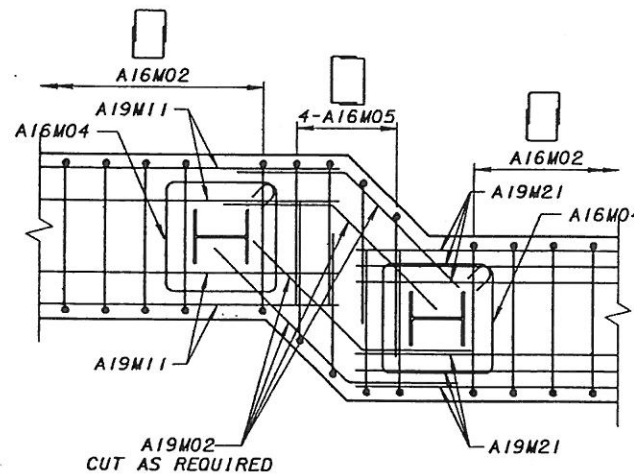
ELEVATION

NOTES:

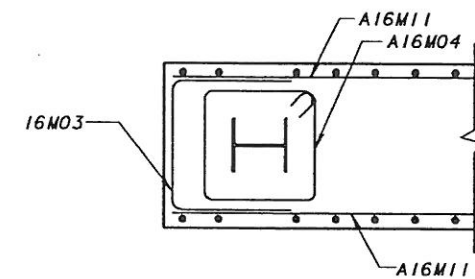
1. FOR SECTIONS A-A, B-B, AND C-C SEE SHEET 6/19



SECTION D-D
(FIXED CONNECTION)



SECTION E-E

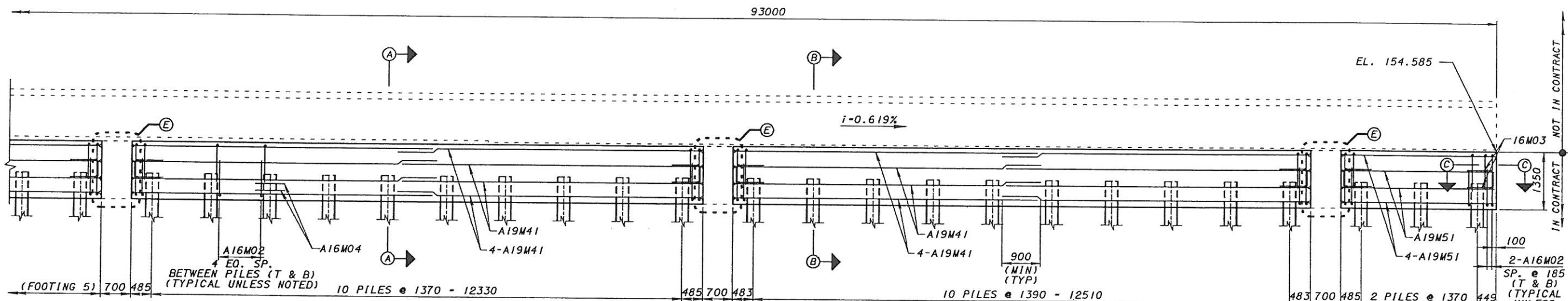
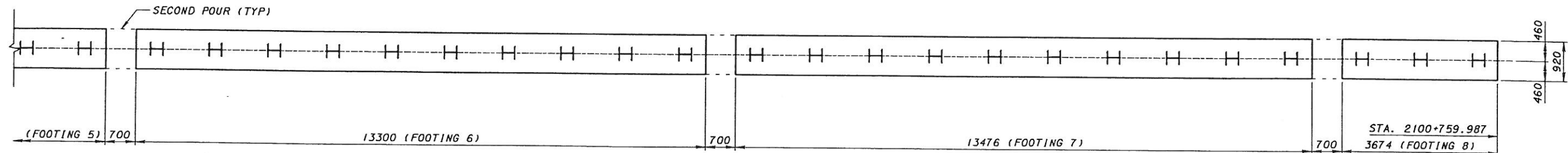


SECTION F-F

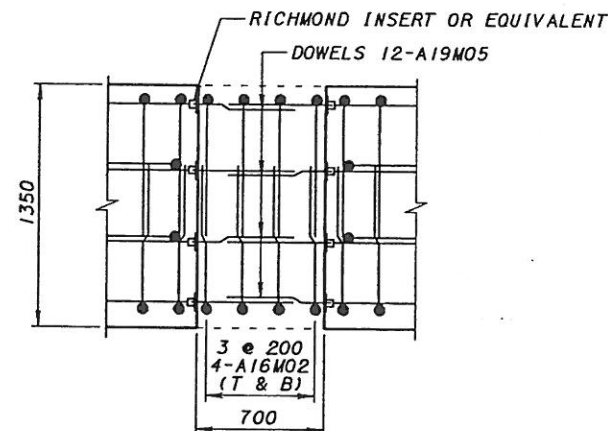
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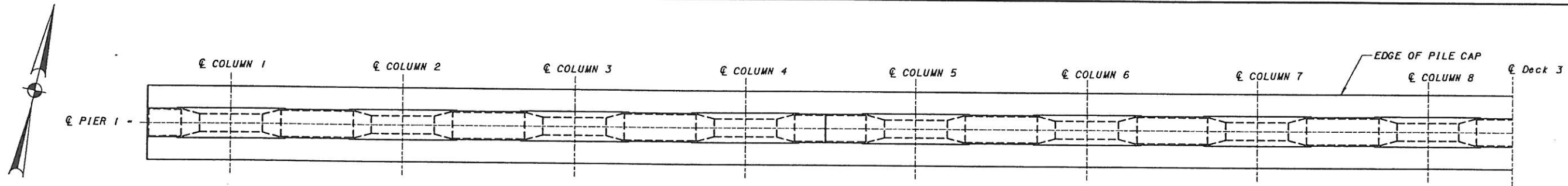
NOTE:
REINFORCEMENT OF FOOTING
4, 5, AND 6 IS IDENTICAL
TO FOOTING 6



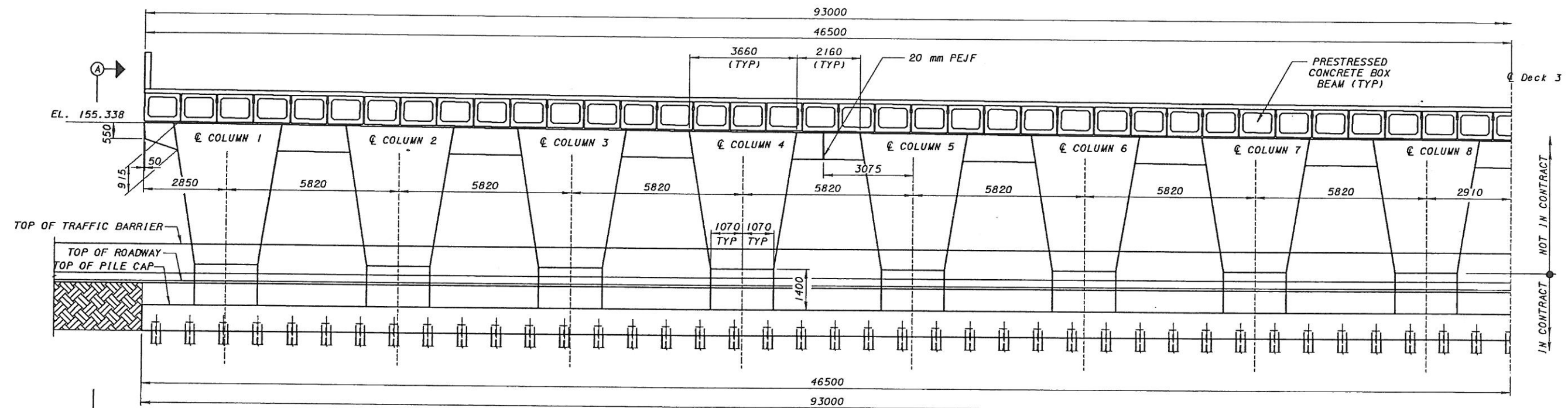
NOTES:
1. FOR SECTIONS A-A, B-B, C-C, AND D-D SEE SHEET 8/19

DESIGN AGENCY PARSONS BRINCKERHOFF, INC. 312 ELM STREET SUITE 2500 CINCINNATI, OH 45202-8720 5025	DATE	6-30-00
	REVISED	STRUCTURE FILE NUMBER
	DRAWN	SRL
	CHECKED	JSH
HAMILTON COUNTY STA. 100+666.987 STA. 100+759.987	PREPARED	VR
	CHECKED	JSH
SOUTH ABUTMENT REINFORCEMENT (SHEET 2 OF 2) DECK NUMBER 3 OVER FORT WASHINGTON WAY		
CITY OF CINCINNATI CONTRACT NO. 75X5753		
9/19		

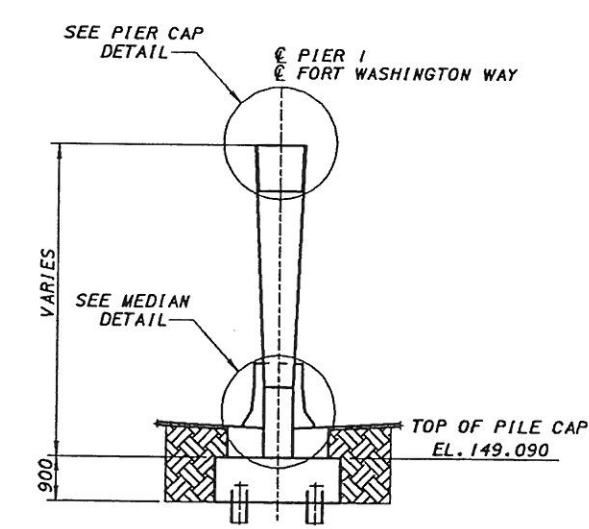
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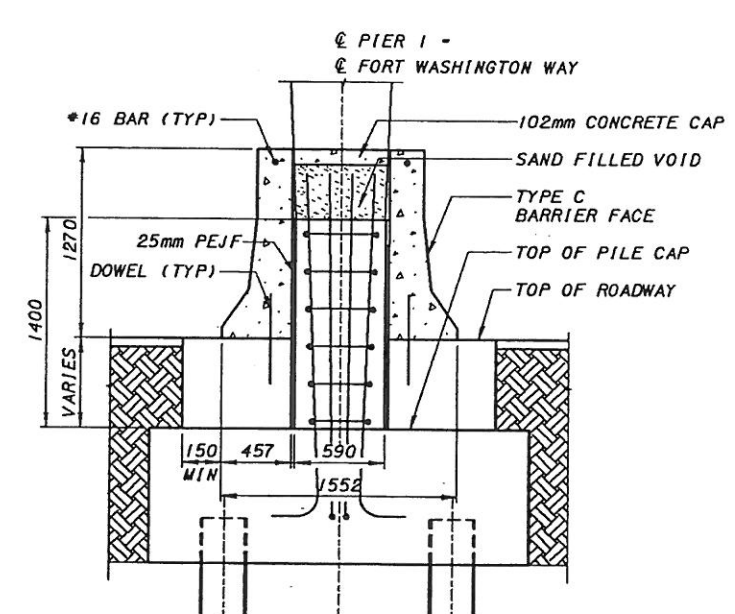
PLAN



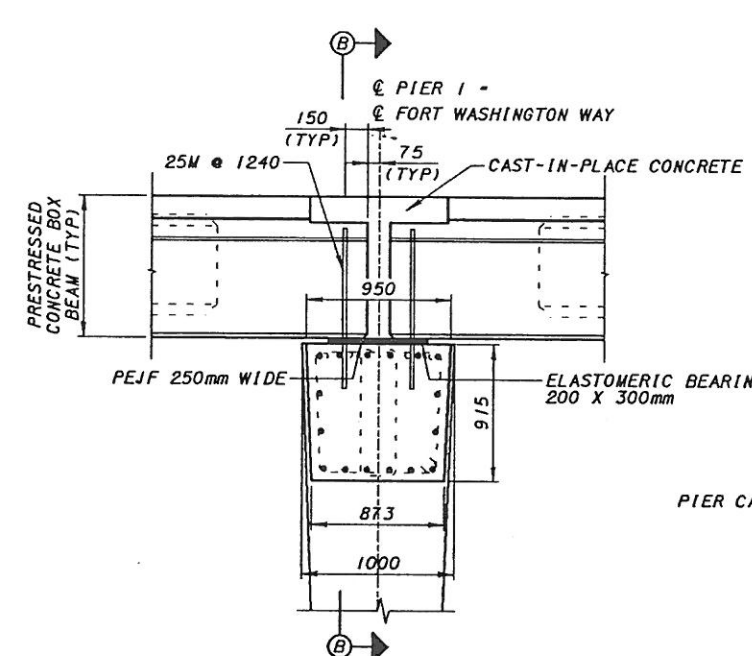
ELEVATION AT PIER 1



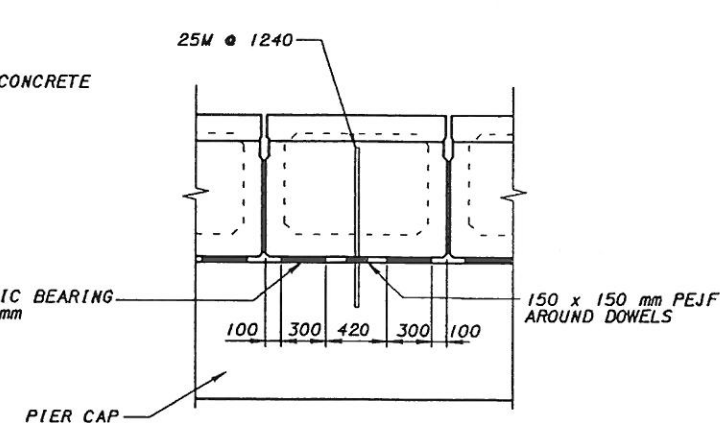
VIEW A-A



MEDIAN DETAIL



PIER CAP DETAIL



SECTION B-B

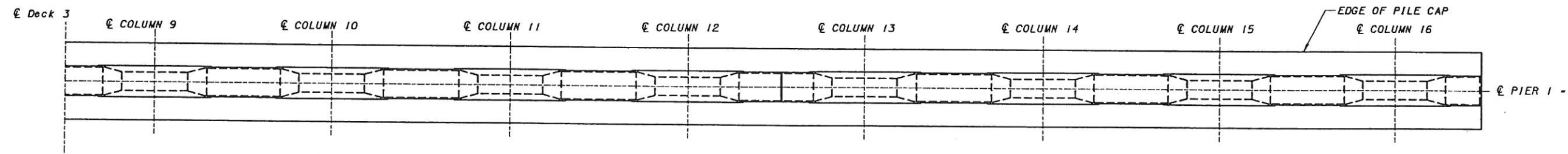
NOTES:
1. SEE NOTES 1 AND 2 ON SHEET 2/19

HAMILTON COUNTY	STA. 100+666.987	PIER 1 PLAN & ELEVATION (1 OF 2)	
	STA. 100+759.987		DECK NUMBER 3
	OVER FORT WASHINGTON WAY		CITY OF CINCINNATI
			CONTRACT NO. 75X5753
		10/19	

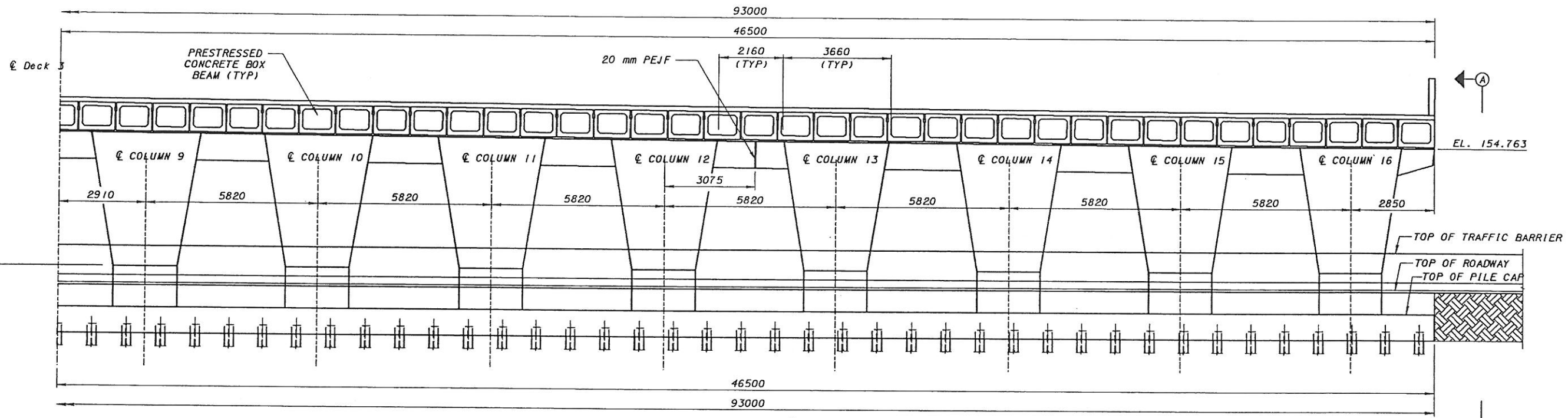
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IN CONTRACT NOT IN CONTRACT



PLAN



ELEVATION AT PIER 1

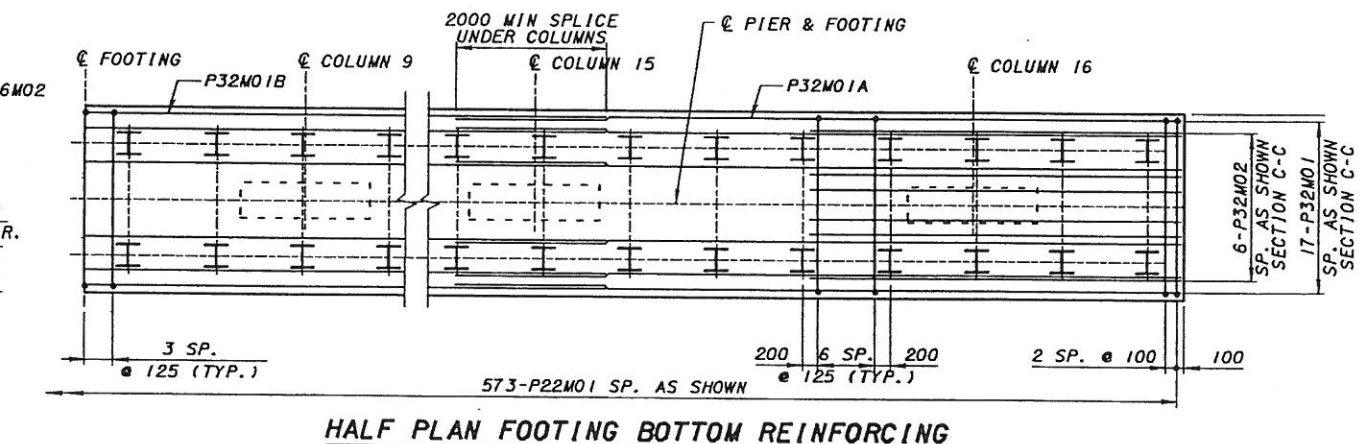
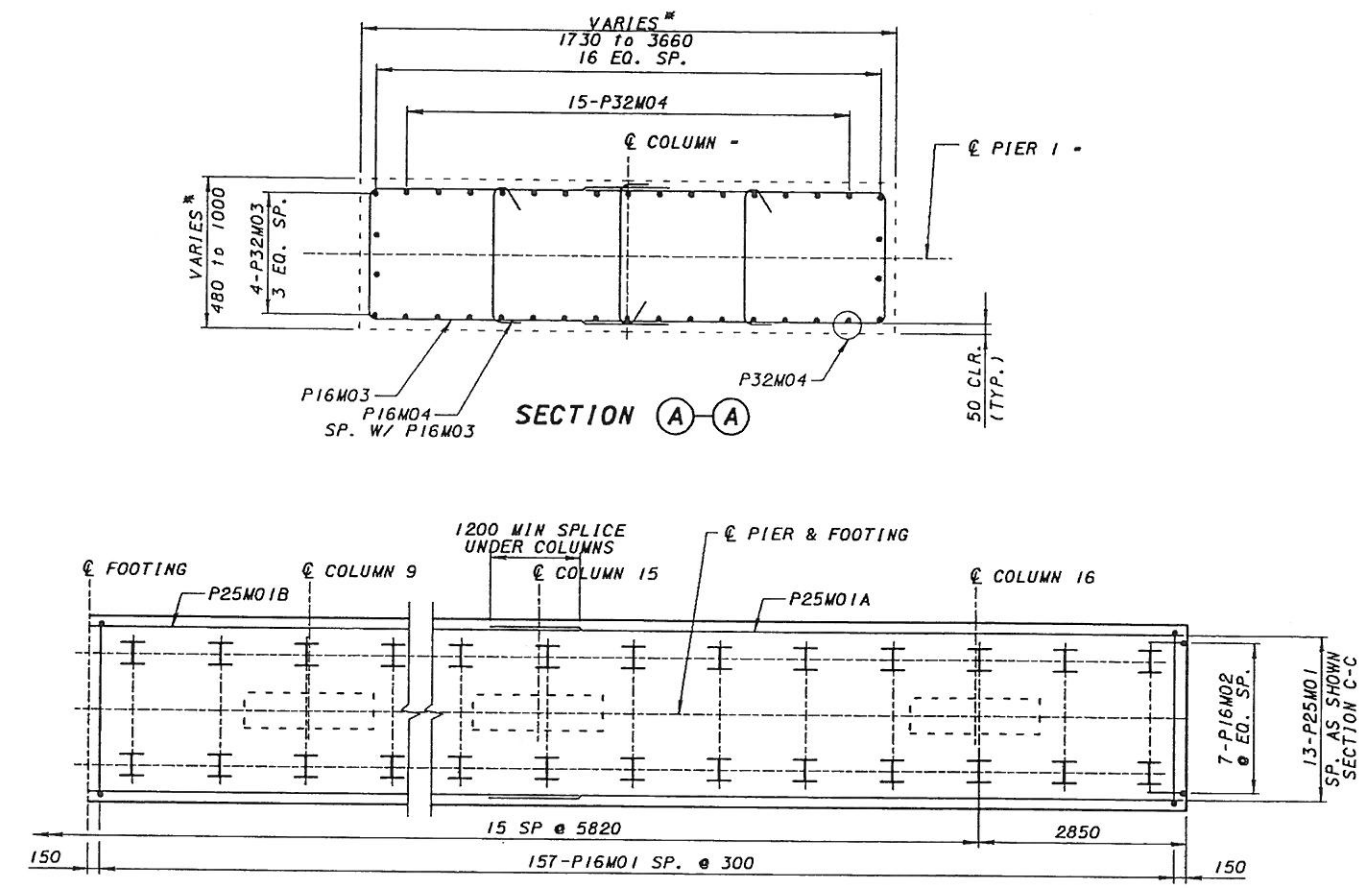
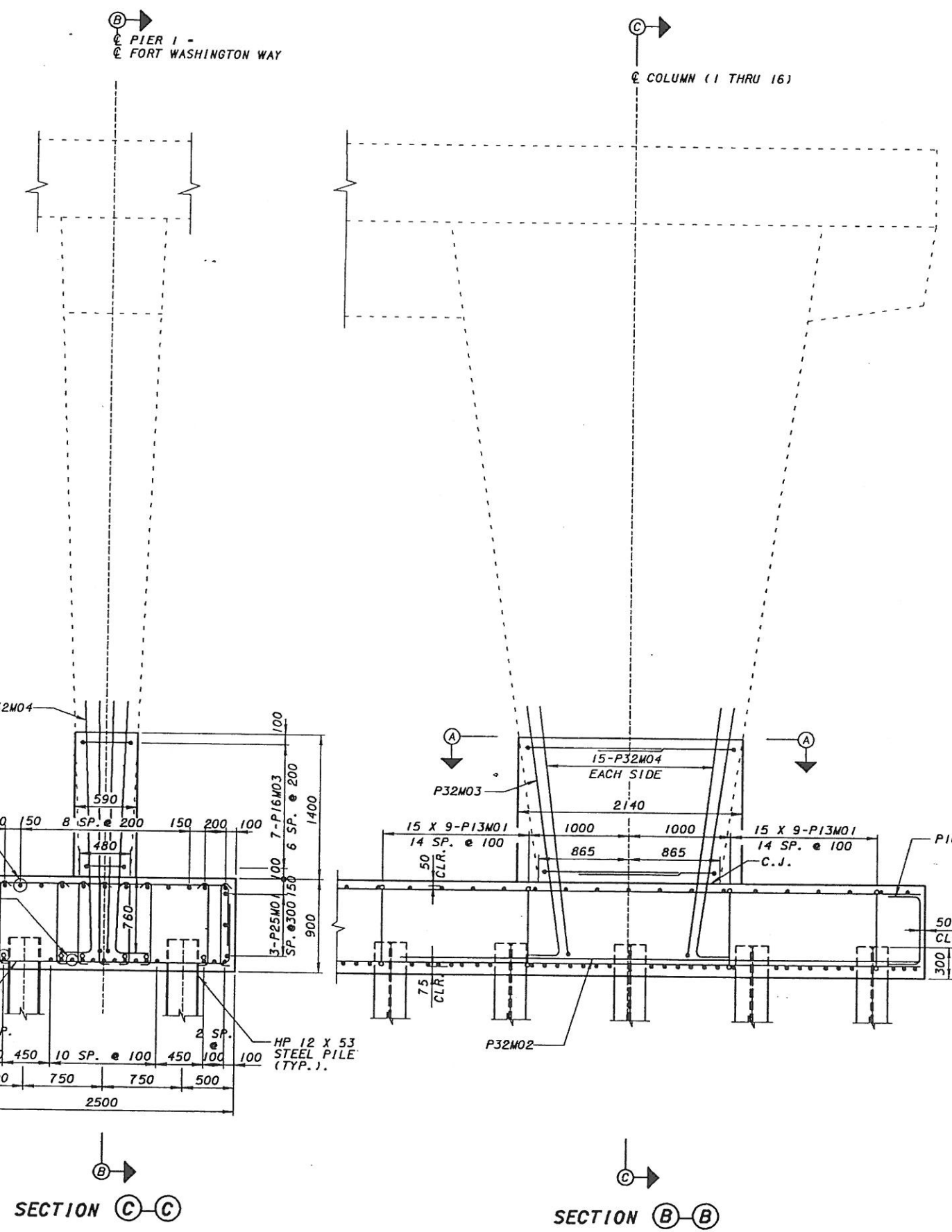
NOTES:
1. FOR SECTION A-A SEE SHEET 10/19

CITY OF CINCINNATI CONTRACT NO. 75X5753	PIER 1 PLAN & ELEVATION (2 OF 2) DECK NUMBER 3 OVER FORT WASHINGTON WAY	HAMILTON COUNTY STA. 100+666.987 STA. 100+759.987	DESIGN AGENCY PARSONS BRINCKERHOFF, INC. CINCINNATI, OH 45202-2720	DATE 6-30-00	REVISED STRUCTURE FILE NUMBER
			DRAWN SRL	REVIEWED	
			PREPARED VR	CHECKED JSH	
			EL. 154.763		

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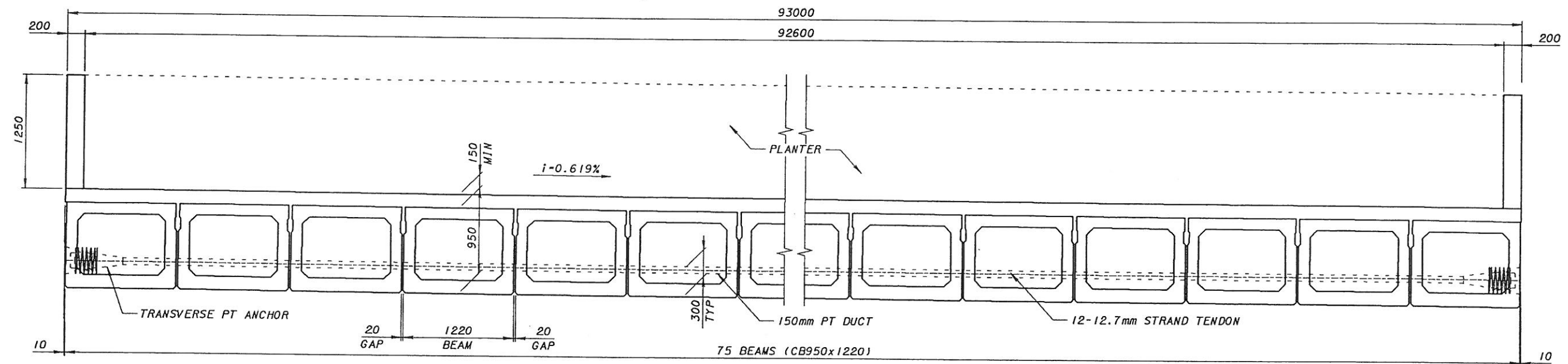
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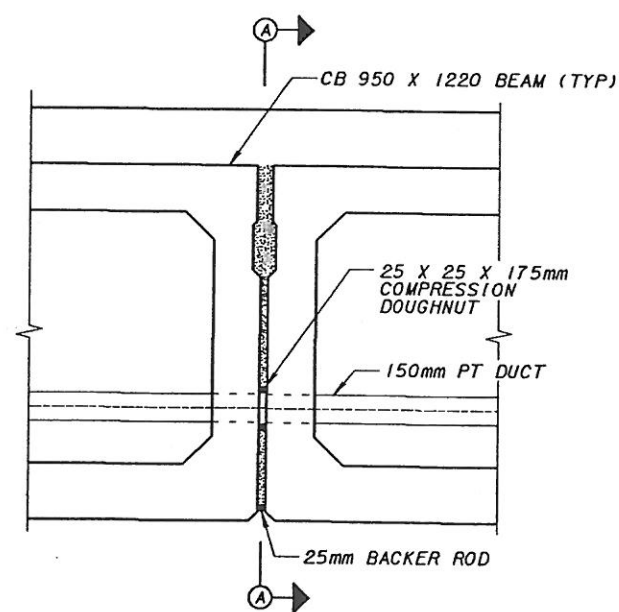


* THE SIZE OF LOWER PORTION OF THE COLUMNS IS 590 x 2140 mm.
THE 480 mm & 1730 mm DIMENSIONS ARE GIVEN TO PROPERLY
LOCATE THE DOWELS IN THE FOOTING.

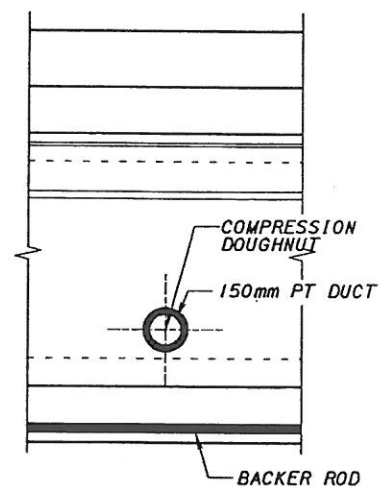
	DESIGN AGENCY	DATE	REVISED	DRAWN	PREPARED	HAMILTON COUNTY STA. 100+666.987 STA. 100+759.987	PIER 1 REINFORCEMENT DECK NUMBER 3 OVER FORT WASHINGTON WAY	CITY OF CINCINNATI CONTRACT NO. 75X5753	12/19
	PARSONS BRINCKERHOFF, INC.	6-30-00	SRL	VR	JSH				
	STRUCTURE FILE NUMBER	REVIEWED	CHECKED						



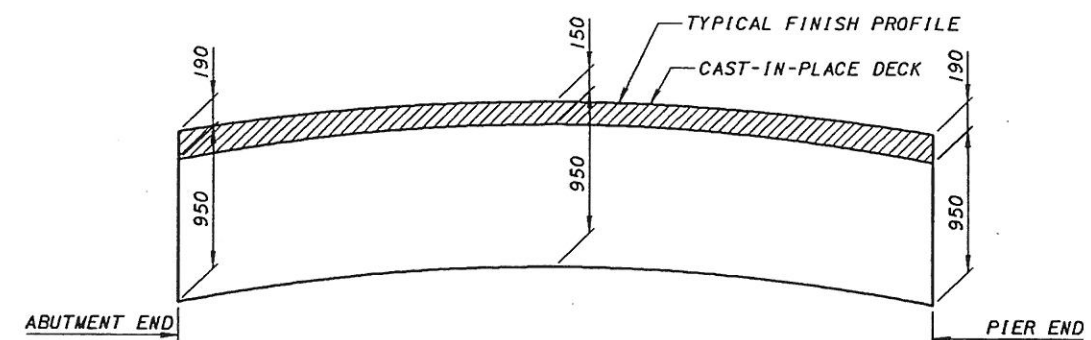
TYPICAL CROSS SECTION



BEAM DETAIL



SECTION 

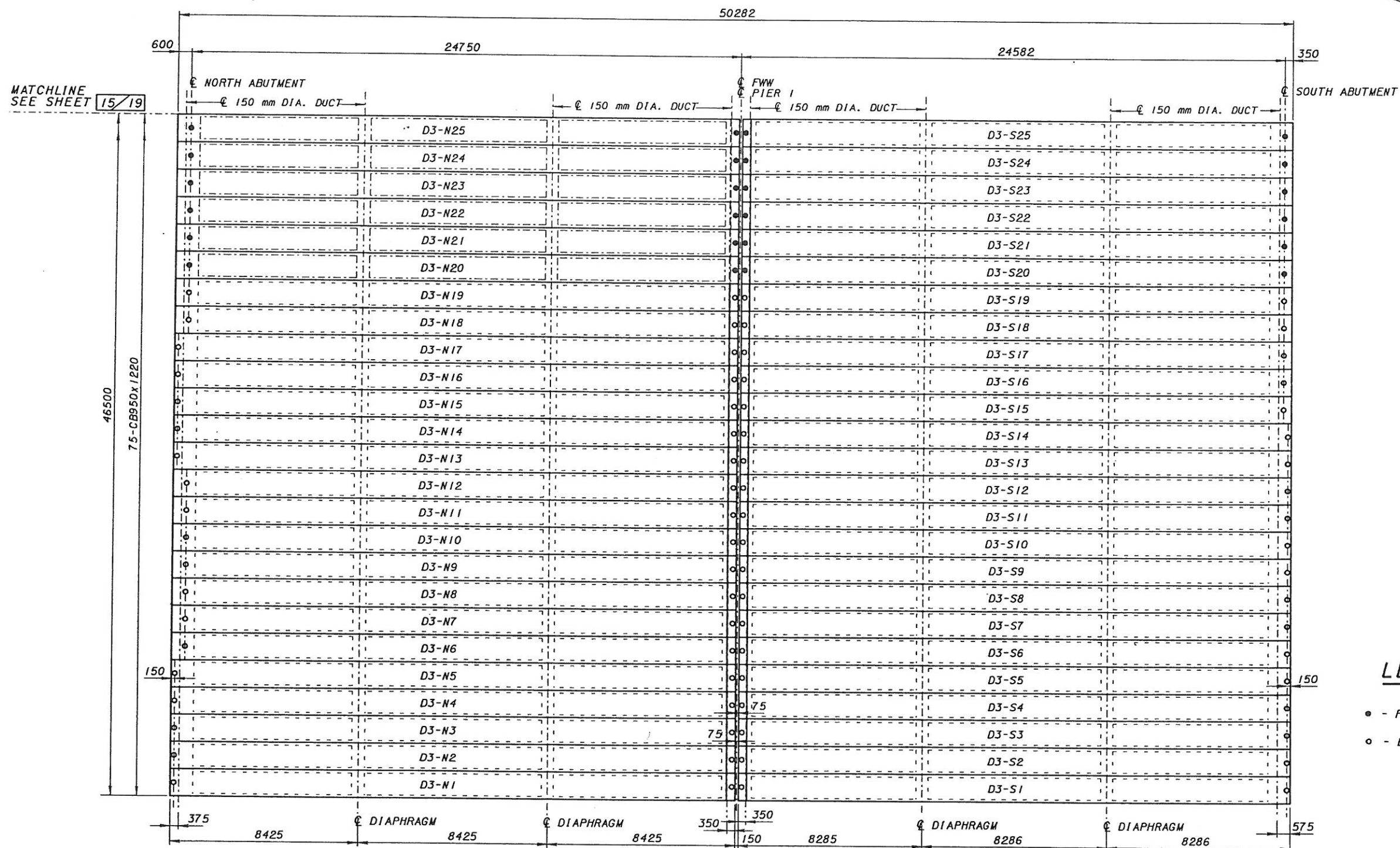


BEAM CAMBER DIAGRAM

NOTES:

1. SEE NOTES 1 AND 2 ON SHEET

2 / 19



FRAMING PLAN

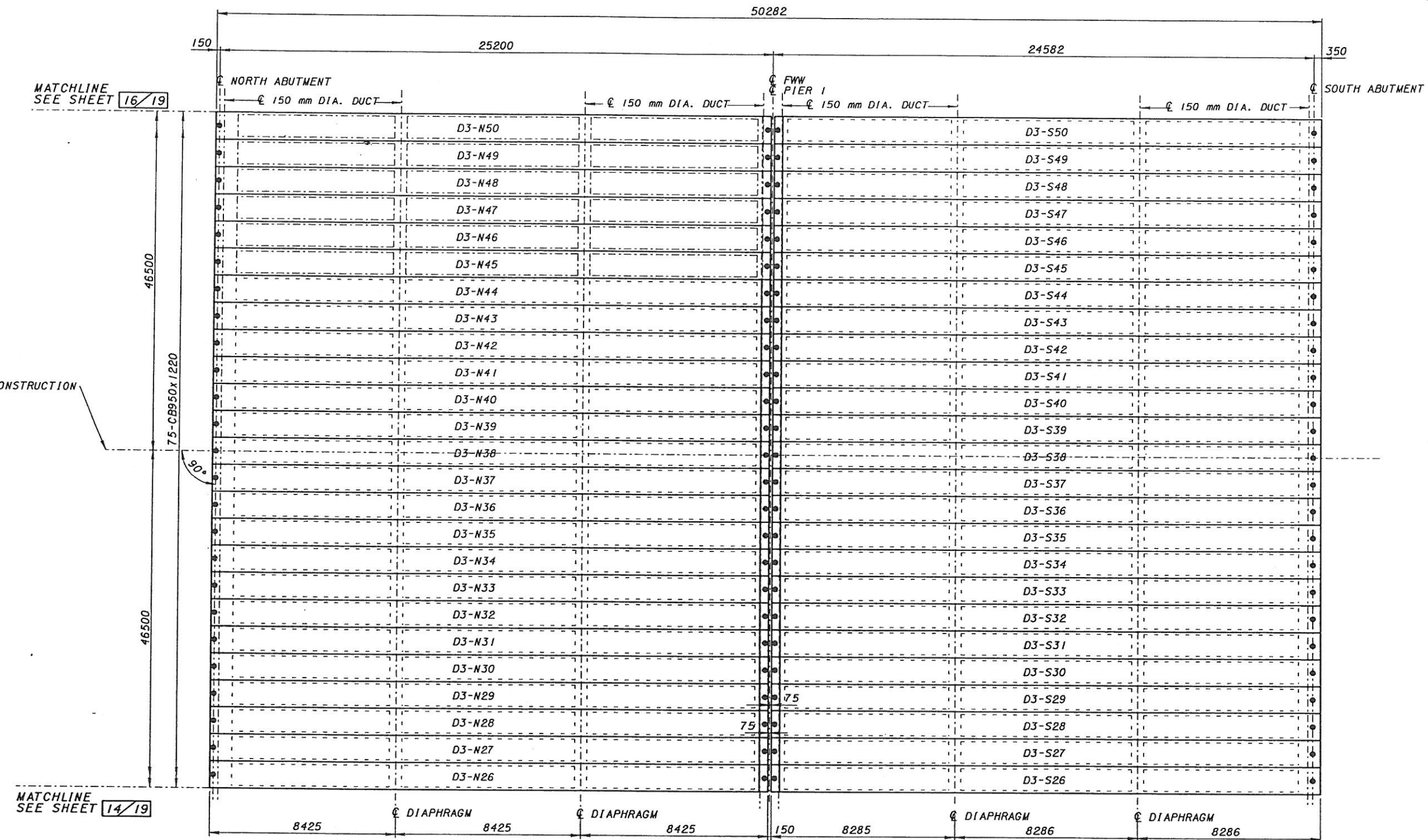
NOTES:

1. MINIMUM CONCRETE STRENGTH OF BEAMS AFTER 28 DAYS: 58.7 MPa, MINIMUM CONCRETE STRENGTH AT RELEASE: 37.2 MPa.
2. PRESTRESSING STRAND TO BE UNCOATED, LOW RELAXATION 12.7 mm DIA., SEVEN WIRES (A_s = 108 sq. mm), ASTM A416, GRADE 270, f_s = 1860 MPa, INITIAL STRESS = 0.75 f_s = 1400 MPa.
3. REINFORCING STEEL TO BE ASTM A615M OR A617M GRADE 400.
4. TOP OF BEAMS ARE TO BE SCORED TRANSVERSELY AT ABOUT 75 mm CENTERS WITH A POINTED TOOL (MAX. DEPTH OF SCORING TO BE 6 mm).
5. OMIT KEYWAY ON EXTERIOR BEAMS.
6. SEE NOTES 1 AND 2 ON SHEET 2/19

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1/12/01

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FRAMING PLAN

NOTES:

1. WORK TOGETHER WITH FRAMING PLAN SHEET 14/19 AND 16/19



DESIGN AGENCY
PARSONS BRINCKERHOFF, INC.
312 ELM STREET, SUITE 2500
CINCINNATI, OH 45202-2720

DATE
6-30-00
REVISED
STRUCTURE FILE NUMBER

DRAWN
SRL
REVIEWED
JSH

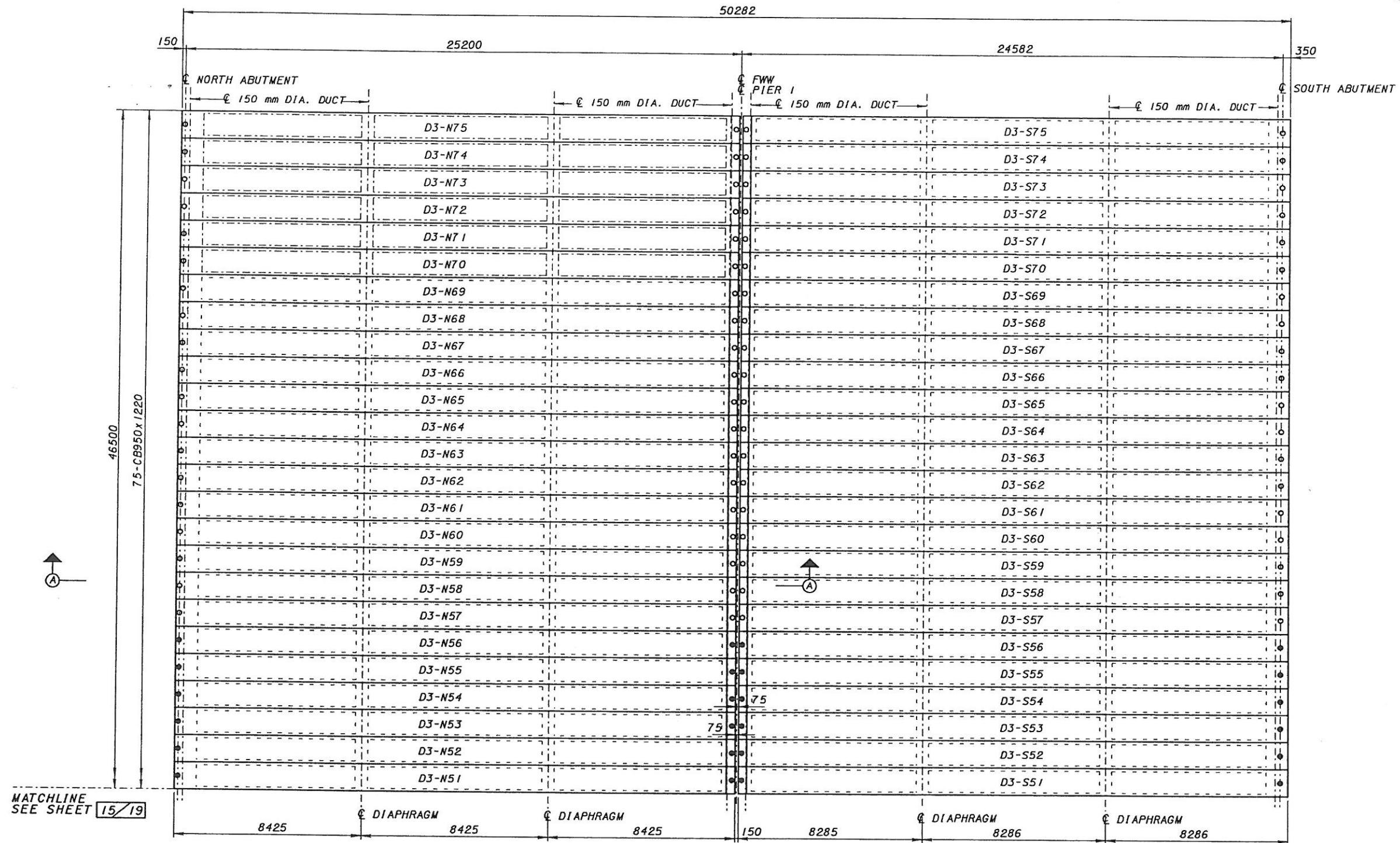
HAMILTON COUNTY
STA. 100+666.987
STA. 100+759.987

FRAMING PLAN (2 OF 3)
DECK NUMBER 3
OVER FORT WASHINGTON WAY

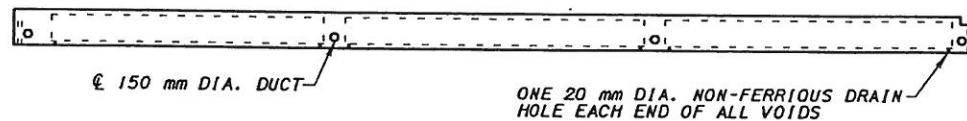
CITY OF CINCINNATI
CONTRACT NO. 75X5753

15/19





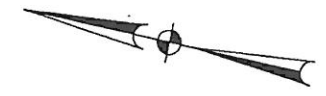
FRAMING PLAN

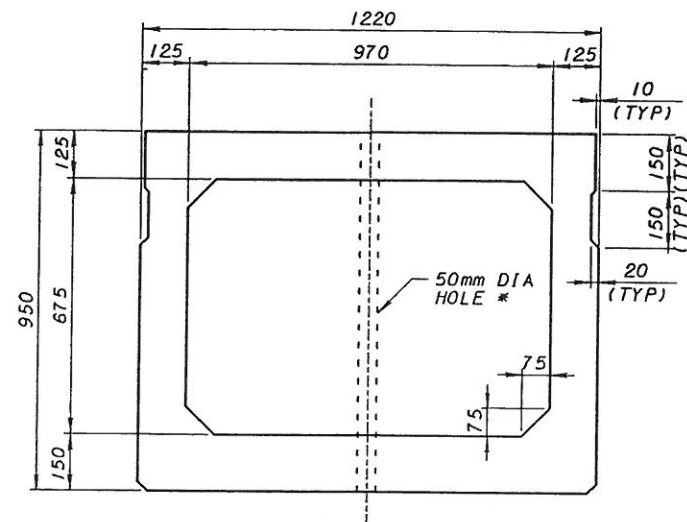


SECTION A-A

NOTES:

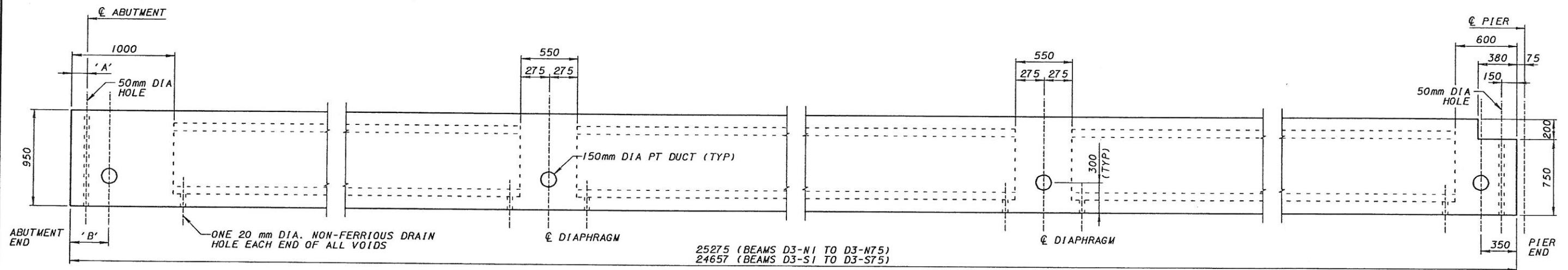
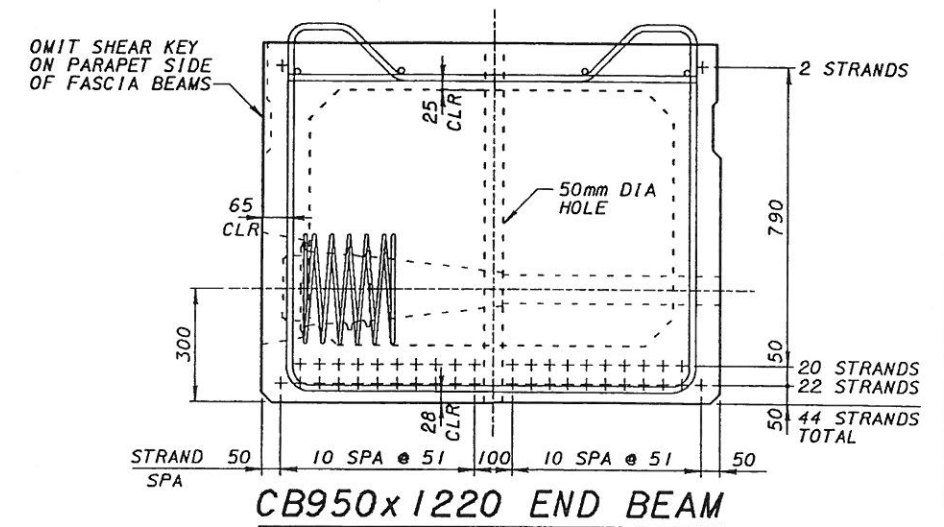
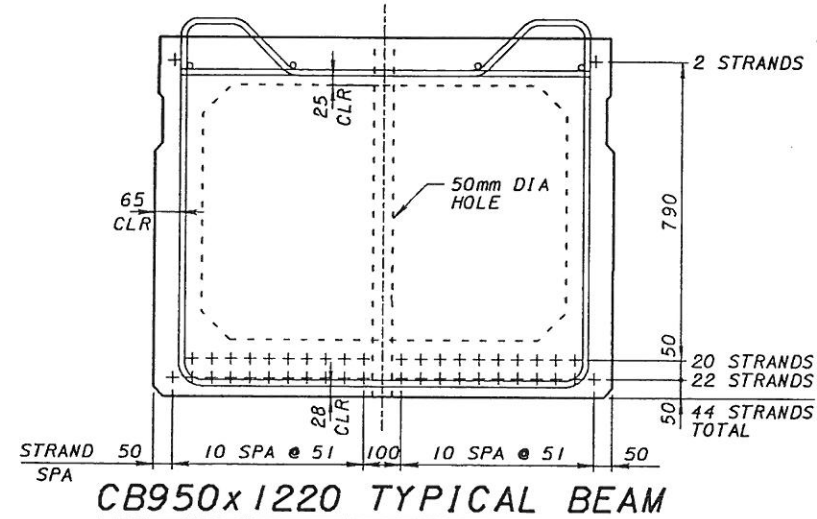
1. WORK TOGETHER WITH FRAMING PLAN SHEET 14/19 AND 15/19



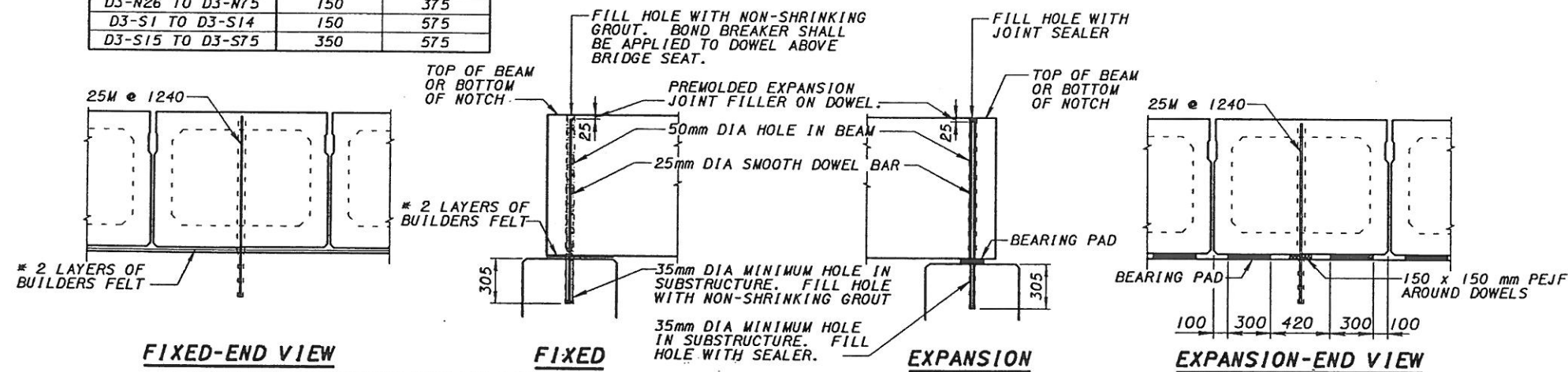


CROSS SECTION CB950x1220 BEAM

* FOR 25mm DIA SMOOTH DOWEL (L-1340)

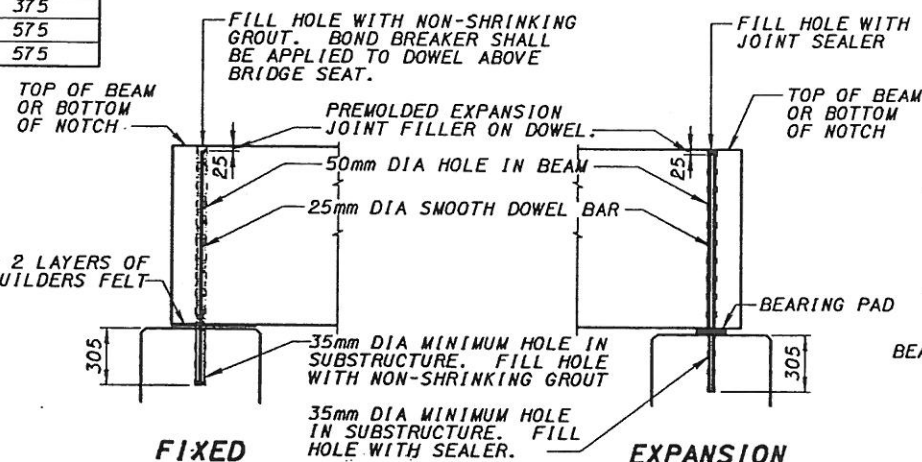


GIRDER VARIABLES		
GIRDER	DIM 'A'	DIM 'B'
D3-N1 TO D3-N5	150	375
D3-N6 TO D3-N12	600	375
D3-N13 TO D3-N17	150	375
D3-N18 TO D3-N25	600	375
D3-N26 TO D3-N75	150	375
D3-S1 TO D3-S14	150	575
D3-S15 TO D3-S75	350	575

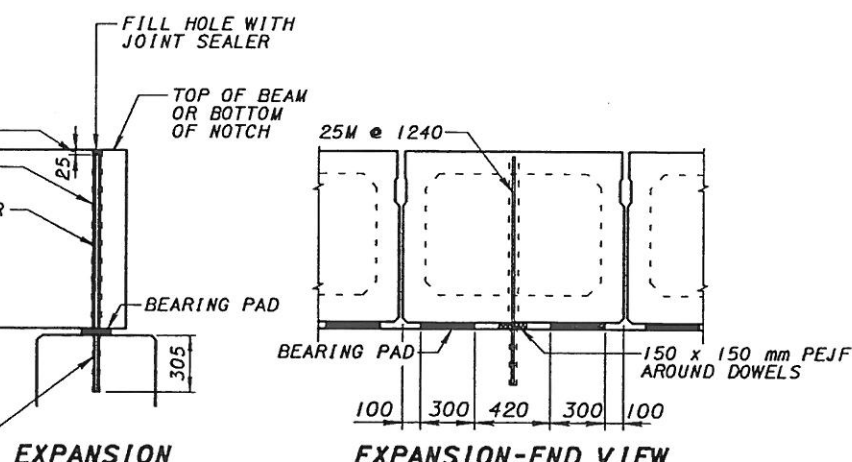


FIXED-END VIEW

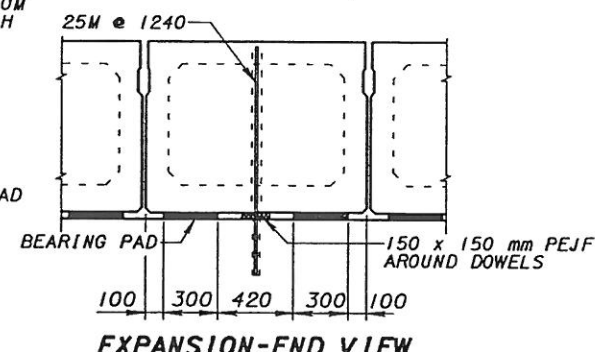
* TWO LAYERS OF BUILDERS FELT ARE USED AT ABUTMENTS IN CONJUNCTION WITH FIXED ANCHOR DOWELS. BEARING PADS SHALL BE USED AT CENTRAL PIER IN CONJUNCTION WITH FIXED ANCHOR DOWELS.



FIXED

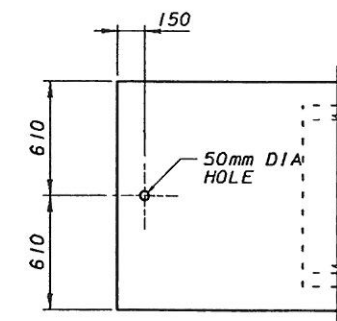


EXPANSION



EXPANSION-END VIEW

NOTE: DOWEL CLEARS ELASTOMERIC BEARING

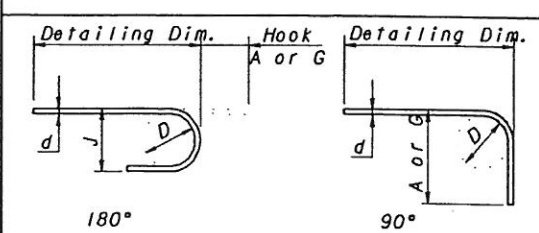
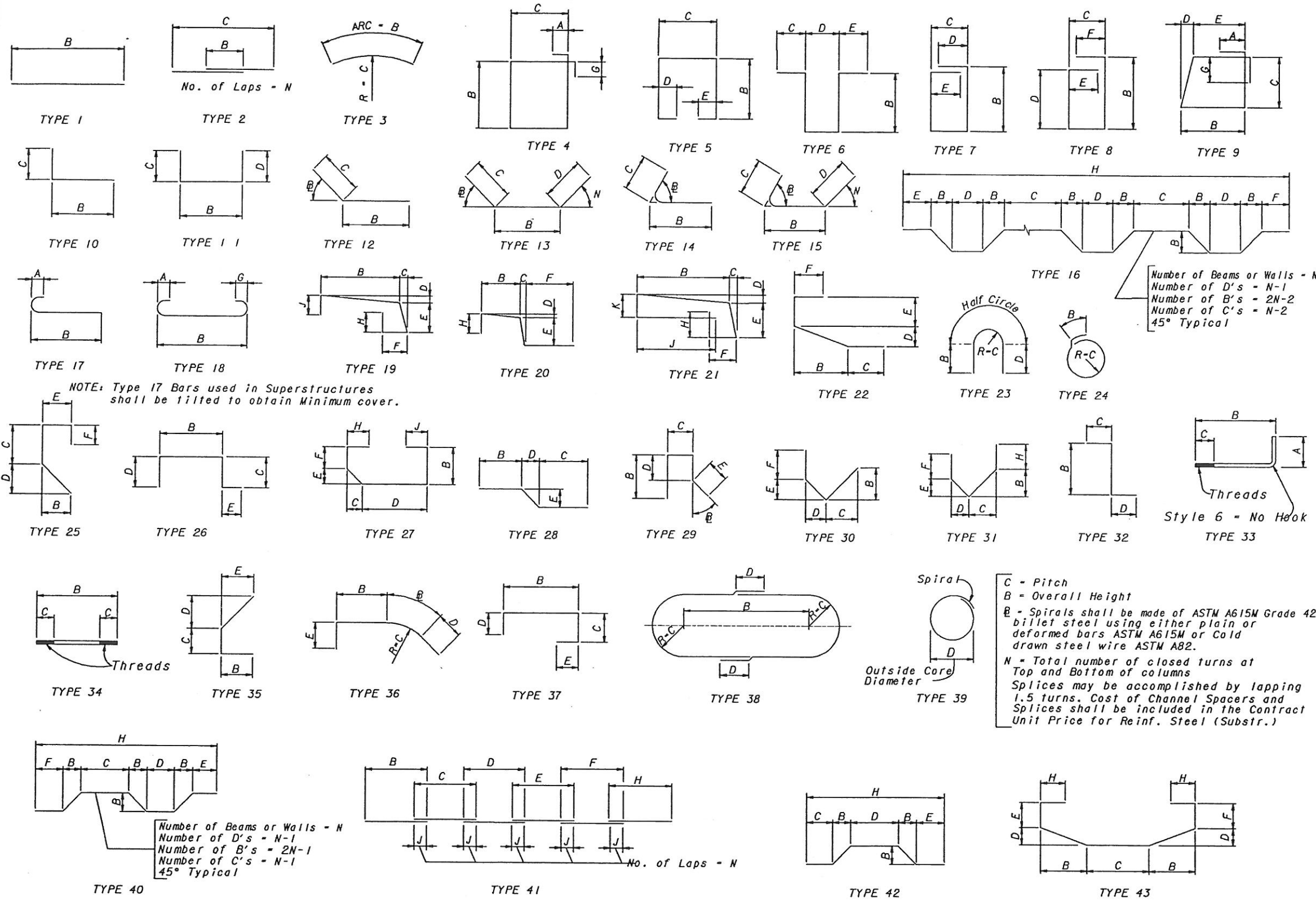


DOWEL HOLE LOCATION

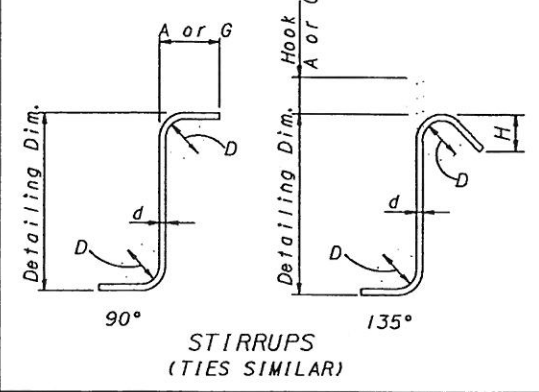
NOTES:

1. SEE NOTES 1 AND 2 ON SHEET

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BAR SIZE	D	180° HOOKS		90° HOOKS	
		A OR G	J	A OR G	
#10	60	125	80	150	
#13	80	150	105	200	
#16	95	175	130	250	
#19	115	200	155	300	
#22	135	250	180	375	
#25	155	275	205	425	
#29	240	375	300	475	
#32	275	425	335	550	
#36	305	475	375	600	
#43	465	675	550	775	
#57	610	925	725	1050	
STYLE		I		3	



BAR SIZE	D	90° HOOKS		135° HOOKS	
		A or G	A or G	H*	
#10	40	105	105	65	
#13	50	115	115	80	
#16	65	155	140	95	
#19	115	305	205	115	
#22	135	355	230	135	
#25	155	410	270	155	
STYLE		4		5	

STYLE 6 - NO HOOK

* Dimension is approximate.

Hook Styles Detailed on this sheet are for Illustration Only.

Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet.

All dimensions are in millimeters (mm), except as noted.

All Dimensions are out-to-out.

NOTE: For Bar Dimensions See REINFORCING BAR LIST Sheet.

MARK	TOTAL	LENGTH	WEIGHT	TYPE	STYLE		DIMENSIONS (mm)										NO	ANG	REMARKS
	NUMBER	(mm)	(kg)		A	G	B	C	D	E	F	H	J	K	N	O			
PIER FOOTING																			
P13M01	4320	1005	4316	18	4	5	775												
P16M01	314	3600	1754	11			2400	600	600										
P16M02	14	1335	29	11			735	300	300										
P22M01	573	3600	6275	11			2400	600	600										
P25M01-A	19	91260	6890	2			1200	76860							12				
P25M01-B	38	9570	1445	1			9170												
P32M01-A	17	101660	11068	2			2000	77660							12				
P32M01-B	34	9570	2084	1			9570												
P32M02	12	5000	384	1			5000												
COLUMN STUB																			
P16M03	32	3635		11			495	1570	1570										
	SETS OF	T0	1245				T0												
P16M03	7	3525		11			385	1570	1570										
P16M04	48	790		18	4	5	495												
	SETS OF	T0	383				T0												
P16M04	7	680		18	4	5	385												
P32M03	128	3050	2500	12			2750	300								98			
P32M04	480	3050	9375	12			2750	300								92			
NORTH ABUTMENT																			
A16M02	694	2450	2639	11			820	815	815										
A16M03	4	2085	13	11			785	650	650										
A16M04	126	3420	669	4	5	5	785	785											
A16M05	32	2895	144	11			1225	835	835										
A19M02	48	1600	172	12			600	1000								45			
A19M11	12	6100	164	1			6100												
A19M21	20	8580	384	1			8580												
A19M31	12	6100	164	1			6100												
A19M41	12	9820	263	1			9820												
A19M42	2	5000	22	1			5000												
A19M51	12	66400	1781	2			900	61900							5				
SOUTH ABUTMENT																			
A16M02	646	2450	2456	11			820	815	815										
A16M03	32	2085	104	11			785	650	650										
A16M04	132	3420	701	4	5	5	785	785											
A16M05	2	2895	9	11			1225	835	835										
A19M02	12	1600	43	12			600	1000								45			
A19M11	12	4350	117	1			4350												
A19M21	12	12110	325	1			12110												
A19M31	12	990	27	1			990												
A19M41	48	14100	1513	2			900	13200							1				
A19M41	12	14275	383	2			900	13375							1				
A19M51	12	3570	96	1			3570												

NOTES:

1. ALL DIMENSIONS ARE OUT TO OUT OF BAR EXCEPT ON STANDARD 90°, 135° AND 180° HOOKS.
2. DIMENSIONS ON HOOKS TO BE SHOWN ONLY WHERE NECESSARY TO RESTRICT HOOK SIZE, OTHERWISE STANDARD HOOKS ARE TO BE USED.
3. ALL BENDS SHOWN ARE BENT AROUND A STANDARD MANDREL. EXCEPT WHERE RADIUS OR DIAMETER IS INDICATED.
4. ALL BARS ARE EPOXY COATED.
5. SP BARS ARE SPIRAL BARS. THE WEIGHT OF THE SPIRAL INCLUDES 11 KILOGRAMS FOR 3 ADDITIONAL COILS, 1½ COILS AT EACH END PER AASHTO 8.18.2.2.4 THE WEIGHT OF THE SPIRAL INCLUDES 13.6 KILOGRAMS FOR 3 ADDITIONAL COILS. 1½ COILS AT EACH END PER AASHTO 8.18.2.2.4. CONCRETE SPACERS OR OTHER APPROVED NON-CORROSIVE SPACING DEVICES SHALL BE USED AT SUFFICIENT INTERVALS (NEAR THE BOTTOM AND AT INTERVALS NOT EXCEEDING 3.050m) TO INSURE CONCENTRIC SPACING FOR THE ENTIRE CAGE LENGTH. SPACERS SHALL BE CONSTRUCTED OF APPROVED MATERIAL EQUAL IN QUALITY AND DURABILITY TO THE CONCRETE SPECIFIED FOR THE COLUMN. THE SPACERS SHALL HAVE ADEQUATE DIMENSIONS TO ENSURE A MINIMUM 75mm CLEAR SPACE BETWEEN THE OUTSIDE OF THE REINFORCING CAGE AND THE DESIGN DIMENSION OF THE COLUMN. CYLINDRICAL CONCRETE FEET (BOTTOM SUPPORTS) SHALL BE PROVIDED TO ENSURE THAT THE BOTTOM OF THE CAGE IS MAINTAINED AT THE PROPER DISTANCE ABOVE THE BASE.
6. THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST TWO DIGITS INDICATE THE BAR SIZE NUMBER. FOR EXAMPLE, P19M01 IS A NO. 19M BAR.
7. FOR STYLE OF HOOK "A" OR "G" AND HOOK DIMNESIONS REFER TO THE STANDARD BAR BENDING DETAILS. TOTAL LENGTH AND WEIGHT OF BARS INCLUDE HOOKS.