


TITLE	SHEET NO.
SITE PLANS	
INDEX OF SHEETS	1 / 108
SITE PLAN 1/2	2 / 108
SITE PLAN 2/2	3 / 108
GENERAL PLAN	
GENERAL PLAN 1/3	4 / 108
GENERAL PLAN 2/3	5 / 108
GENERAL PLAN 3/3	6 / 108
GENERAL NOTES	
ESTIMATED QUANTITIES & GENERAL NOTES 1/5	7 / 108
GENERAL NOTES 2/5	8 / 108
GENERAL NOTES 3/5	9 / 108
GENERAL NOTES 4/5	10 / 108
GENERAL NOTES 5/5	11 / 108
MISCELLANEOUS DETAILS	12 / 108
ABUTMENTS	
PORTIONS OF STRUCTURES REMOVED REAR ABUTMENT (ABUTMENT No. 1)	13 / 108
REAR ABUTMENT MODIFICATION (ABUTMENT No. 1)	14 / 108
PORTIONS OF STRUCTURES REMOVED FORWARD ABUTMENT (ABUTMENT No. 18)	15 / 108
FORWARD ABUTMENT MODIFICATION (ABUTMENT No. 18)	16 / 108
PIERS	
PATCHING CONCRETE STRUCTURES PIERS (PIER NO. 1 TO PIER NO. 5)	17 / 108
PATCHING CONCRETE STRUCTURES PIERS (PIER NO. 6 TO PIER NO. 9)	18 / 108
PATCHING CONCRETE STRUCTURES PIERS (PIER NO. 10 TO PIER NO. 11)	19 / 108
PATCHING CONCRETE STRUCTURES PIERS (PIER NO. 12 TO PIER NO. 14)	20 / 108
PATCHING CONCRETE STRUCTURES PIERS (PIER NO. 15 TO PIER NO. 17)	21 / 108
CONCRETE REMOVALS	
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 1/3 UNITS A-F	22 / 108
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 2/3 UNITS F-I	23 / 108
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 3/3 UNITS I-M	24 / 108
STRUCTURAL STEEL REMOVALS	
STRUCTURAL STEEL DETAILS REMOVALS 1/3 UNITS A-F	25 / 108
STRUCTURAL STEEL DETAILS REMOVALS 2/3 UNITS F-I	26 / 108
STRUCTURAL STEEL DETAILS REMOVALS 3/3 UNITS I-M	27 / 108
STRUCTURAL STEEL REPLACEMENT OF DETERIORATED END CROSS FRAMES (PLATE GIRDER SPANS)	28 / 108
STRUCTURAL STEEL REPLACEMENT OF DETERIORATED END CROSS FRAMES (TRUSS SPANS)	29 / 108
BEARINGS	
BEARING DEVICES SCHEMATIC PLAN 1/3 (REAR ABUTMENT-PIER NO. 7)	30 / 108
BEARING DEVICES SCHEMATIC PLAN 2/3 (PIER NO. 7-PIER NO. 12)	31 / 108
BEARING DEVICES SCHEMATIC PLAN 3/3 (PIER NO. 12-FORWARD ABUTMENT)	32 / 108
EXISTING CAST STEEL BEARING DEVICES DETAIL	33 / 108
ELASTOMERIC BEARING RETROFIT DETAILS	34 / 108
SHTS. NOT USED	35 / 108
TRUSS BEARING DEVICES PROPOSED POT BEARING	36 / 108
TRUSS BEARING DEVICES SETTING SCHEDULE POT BEARINGS	37 / 108

TITLE	SHEET NO.
STUDS	
STRUCTURAL STEEL DETAILS STUD SHEAR CONNECTOR SPANS 1-2 THRU 4-5 (UNITS A THRU D)	38 / 108
STRUCTURAL STEEL DETAILS STUD SHEAR CONNECTOR SPANS 5 THRU 12 (UNITS E, F, & G)	39 / 108
STRUCTURAL STEEL DETAILS STUD SHEAR CONNECTOR SPANS 12-13 THRU SPANS 17-18 (UNITS H THRU M)	40 / 108
BRACKETS	
STRUCTURAL STEEL DETAILS NEW BRACKETS SPANS 1-2 THRU 4-5 (SLAB UNITS A, B, C, & D)	41 / 108
STRUCTURAL STEEL DETAILS NEW BRACKETS TRUSS SPANS 5-6-7, 7-8-9-10, & 10-11-12 (SLAB UNITS E, F, & G)	42 / 108
STRUCTURAL STEEL DETAILS NEW BRACKETS SPANS 12-13 THRU 17-18 (SLAB UNITS H, I, J, K, L, & M)	43 / 108
STRUCTURAL STEEL DETAILS SPECIAL TIE PLATES ON NEW BRACKETS SPANS 1-2 THRU 17-18 (UNITS A THRU M)	44 / 108
STRUCTURAL STEEL DETAILS BRACKETS SPANS 1-5 & 12-18 (UNITS A-D THRU H-M)	45 / 108
STRUCTURAL STEEL DETAILS BRACKETS SPANS 5-6-7, 7-8-9-10 & 10-11-12 (UNITS E, F, & G LEFT SIDE)	46 / 108
STRUCTURAL STEEL DETAILS BRACKETS TRUSS SPANS 5-6-7, 7-8-9-10 & 10-11-12 (UNITS E, F, & G RIGHT SIDE)	47 / 108
DRAINAGE	
DECK DRAINAGE PLAN AND DOWNSPOUT DETAILS	48 / 108
DRAINAGE DETAILS TYPICAL INLET	49 / 108
DRAINAGE DETAILS INLET LOCATIONS (REAR ABUT. & PIER NO. 2 AND 3)	50 / 108
DRAINAGE DETAILS INLET LOCATIONS (PIER NO. 4 THRU PIER NO. 10)	51 / 108
DRAINAGE DETAILS INLET LOCATIONS (PIER NO. 12 THRU PIER NO. 17)	52 / 108
EXPANSION JOINTS	
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT REAR ABUTMENT (No. 1) AND AT PIER No. 2	53 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 3 AND RAMP A	54 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 4 AND PIER No. 5	55 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 7 AND PIER No. 10	56 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 12 AND 6th STREET CONNECTION	57 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 13, No. 14, No. 15, AND No. 16	58 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No. 17 AND AT FORWARD ABUTMENT (No. 18)	59 / 108
DECK SLABS	
SPAN 1-2 DECK SLAB UNIT A	60 / 108
SPAN 2-3 DECK SLAB UNIT B	61 / 108
SPAN 3-4 DECK SLAB UNIT C	62 / 108
SPAN 4-5 DECK SLAB UNIT D	63 / 108

TITLE	SHEET NO.
SPAN 5-6-7 DECK SLAB UNIT E	64 / 108
SPAN 7-8-9-10 DECK SLAB UNIT F 1/2	65 / 108
SPAN 7-8-9-10 DECK SLAB UNIT F 2/2	66 / 108
SPAN 10-11-12 DECK SLAB UNIT G	67 / 108
SPAN 12-13 DECK SLAB UNIT H	68 / 108
SPAN 13-14 DECK SLAB UNIT I	69 / 108
SPAN 14-15 DECK SLAB UNIT J	70 / 108
SPAN 15-16 DECK SLAB UNIT K	71 / 108
SPAN 16-17 DECK SLAB UNIT L	72 / 108
SPAN 17-18 DECK SLAB UNIT M	73 / 108
RAMPS (A & 6th ST.)	
RAMP A DECK SLAB MODIFICATION	74 / 108
SIXTH STREET CONNECTION SLAB MODIFICATION	75 / 108
DECORATIVE RAILING	
DECORATIVE RAILING MISCELLANEOUS DETAILS	76 / 108
DECORATIVE RAILING SPANS 1-2, 2-3, 3-4 & 4-5 (DECK SLAB UNITS A, B, C & D)	77 / 108
DECORATIVE RAILING SPANS 5-6-7 & 7-8-9-10 (UNIT E & UNIT F)	78 / 108
DECORATIVE RAILING SPANS 10-11-12, 12-13, 13-14 & 14-15 (SLAB UNITS G, H, I & J)	79 / 108
DECORATIVE RAILING SPANS 15-16, 16-17 & 17-18 (SLAB UNITS K, L & M)	80 / 108
DECORATIVE RAILING ENDS OF SPAN PYLONS SCHEDULE (SPAN 1-2 THRU 4-5) (SLAB UNITS A, B, C & D)	81 / 108
DECORATIVE RAILING ENDS OF SPAN PYLONS SCHEDULE (SPANS 5-6-7 & 7-8-9-10) (SLAB UNITS E & F)	82 / 108
DECORATIVE RAILING ENDS OF SPAN PYLONS SCHEDULE (SPANS 10-11-12, 12-13, 13-14 & 14-15) (SLAB UNITS G, H, I & J)	83 / 108
DECORATIVE RAILING ENDS OF SPAN PYLONS SCHEDULE (SPANS 15-16, 16-17 & 17-18) (SLAB UNITS K, L, & M)	84 / 108
VANDAL PROTECTION FENCE	85 / 108
BAR LIST	
REINFORCING STEEL LIST ABUTMENTS	86 / 108
REINFORCING STEEL LIST RAMP A & 6th STREET CONNECTOR	87 / 108
REINFORCING STEEL LIST DECK SPAN 1-2 (UNIT A)	88 / 108
REINFORCING STEEL LIST DECK SPAN 2-3 (UNIT B)	89 / 108
REINFORCING STEEL LIST DECK SPAN 3-4 (UNIT C)	90 / 108
REINFORCING STEEL LIST DECK SPAN 4-5 (UNIT D)	91 / 108
REINFORCING STEEL LIST DECK SPAN 5-6-7 (UNIT E)	92 / 108
REINFORCING STEEL LIST DECK SPAN 7-8-9-10 (UNIT F)	93 / 108
REINFORCING STEEL LIST DECK SPAN 10-11-12 (UNIT G)	94 / 108
REINFORCING STEEL LIST DECK SPAN 12-13 (UNIT H)	95 / 108
REINFORCING STEEL LIST DECK SPAN 13-14 (UNIT I) & DECK SPAN 14-15 (UNIT J)	96 / 108
REINFORCING STEEL LIST DECK SPAN 15-16 (UNIT K) & DECK SPAN 16-17 (UNIT L)	97 / 108
REINFORCING STEEL LIST DECK SPAN 17-18 (UNIT M)	98 / 108
REINFORCING STEEL LIST DECORATIVE RAILING	99 / 108

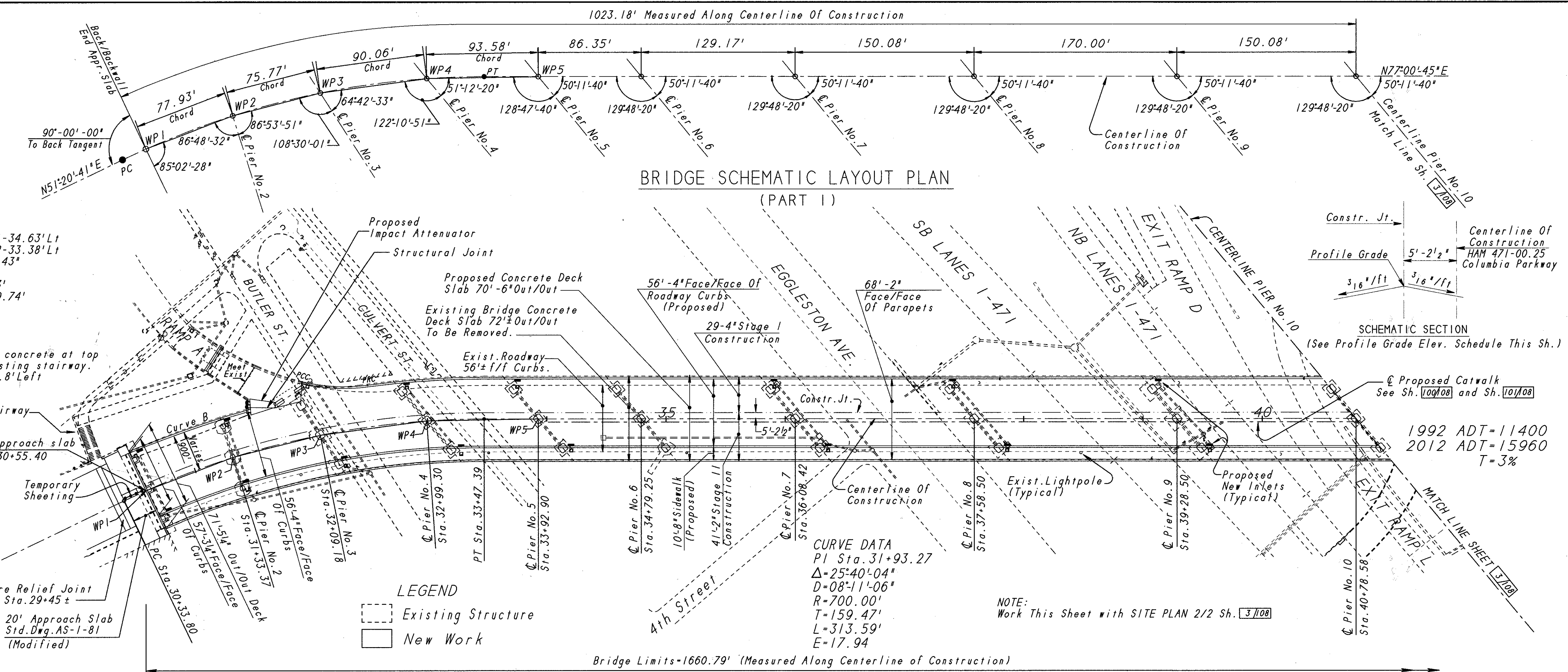
TITLE	SHEET NO.
CATWALK	
CATWALK PLAN & ELEVATION 1/2	100 / 108
CATWALK PLAN & ELEVATION 2/2	101 / 108
CATWALK DETAILS 1/2	102 / 108
CATWALK DETAILS 2/2	103 / 108
CATWALK SERVICE LADDER	
NO. 1 (PIER NO. 5) & NO. 2 (PIER NO. 6)	104 / 108
CATWALK SERVICE LADDER	
NO. 3 (PIER NO. 7) & NO. 4 (PIER NO. 8)	105 / 108
CATWALK SERVICE LADDER	
NO. 5 (PIER NO. 9) & NO. 6 (PIER NO. 10)	106 / 108
CATWALK SERVICE LADDER	
NO. 7 (PIER NO. 11) & NO. 8 (PIER NO. 12)	107 / 108
CATWALK SERVICE LADDER DETAILS	108 / 108

		1 / 108
INDEX OF SHEETS		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	PLF	WDD
CHECKED	REVIEWED	DATE
	IEH	April 96

TITLE Scale 1

HAM-471-00.25

PROFILE GRADE ELEVATIONS SCHEDULE		
LOCATION	STATION, OFFSET	ELEVATION
BEGIN BRIDGE	30+55.24, 5.21' LT	538.55
PIER NO. 2	31+33.37, 5.21' LT	539.39
PIER NO. 3	32+07.13, 5.21' LT	540.57
PIER NO. 4	32+95.57, 5.21' LT	542.29
PIER NO. 5	33+88.56, 5.21' LT	544.60
PIER NO. 6	34+74.91, 5.21' LT	547.28
PIER NO. 7	36+04.08, 5.21' LT	551.55
PIER NO. 8	37+54.16, 5.21' LT	556.52
PIER NO. 9	39+24.16, 5.21' LT	562.15
PIER NO. 10	40+74.24, 5.21' LT	567.12
PIER NO. 11	42+03.41, 5.21' LT	571.39
PIER NO. 12	42+89.91, 5.21' LT	574.25
PIER NO. 13	43+54.30, 5.21' LT	576.38
PIER NO. 14	44+36.95, 5.21' LT	579.12
PIER NO. 15	45+19.60, 5.21' LT	581.86
PIER NO. 16	45+84.60, 5.21' LT	584.01
PIER NO. 17	46+49.60, 5.21' LT	586.16
END BRIDGE	47+16.19, 5.21' LT	588.34



EXISTING STRUCTURE
 STRUCTURE FILE No. 3103390
 TYPE: STRUCTURAL STEEL
 GIRDERS AND TRUSSES AND REINFORCED CONCRETE SUPERSTRUCTURE ON REINFORCED CONCRETE SUBSTRUCTURE.
 ROADWAY: 56' ± f/f of Curbs.
 SIDEWALKS: 8' ± R/L
 SKEW VARIES
 WEARING SURFACE: Concrete.
 ALIGNMENT: Curve Right 700'R then Tangent then Curve Left 850'R then Tangent.
 SPANS: As Noted On Layout Plan
 LOADING: H20-33
 DATE BUILT: 1938

PROPOSED STRUCTURE MODIFICATION
 TYPE: EXISTING STEEL GIRDERS AND TRUSSES AND NEW REINFORCED CONCRETE DECK SUPERSTRUCTURE ON EXISTING REINFORCED CONCRETE SUBSTRUCTURE.
 SPANS: Same As Existing.
 ROADWAY: 56'-4" f/f Of Curbs
 68'-2" f/f Of Parapets
 SIDEWALK: 10'-8" Right Side Only
 LOADING: HS20-44 And Alternate Military
 SKEW: VARIES
 WEARING SURFACE: Monolithic Concrete.
 APPROACH SLAB: 20'-0" Long AS-1-81 Modified
 ALIGNMENT: Existing
 CROWN: 3/16" / F1.

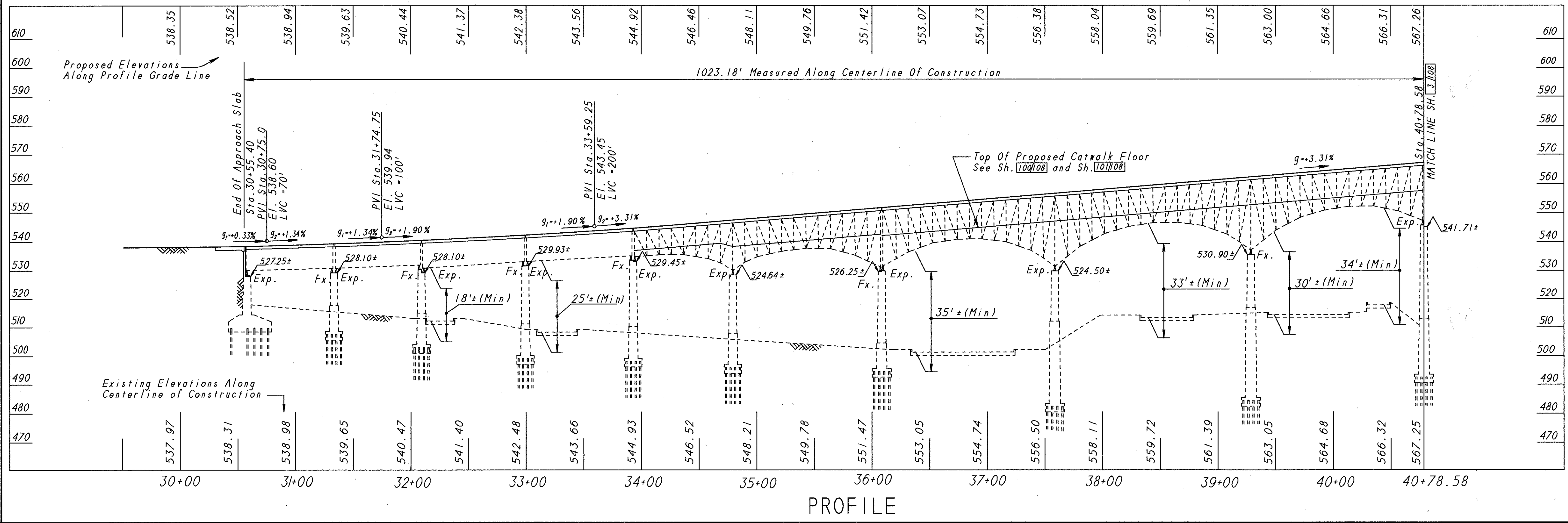
Plum, Klausmeier & Gehrum
 CONSULTANTS

SITE PLAN 1/2

BRIDGE No. HAM-471-0025
 (COLUMBIA PARKWAY VIADUCT OVER I-471)
 Sta. 30+55.40
 Sta. 47+16.19

HAMILTON COUNTY

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISION
ED	ED/PLF		WDD	IEH	April 96	



SITE/STATIONER SCALE 50

PROFILE

REVIEWED BY BURGESS & NIPLE, LIMITED
 JLG/JCS 9-18-96

NOTE:
 WP Denotes 'Working Point'

NOTE:
 Work This Sheet with SITE PLAN 2/2 Sh. 3/108

PLAN

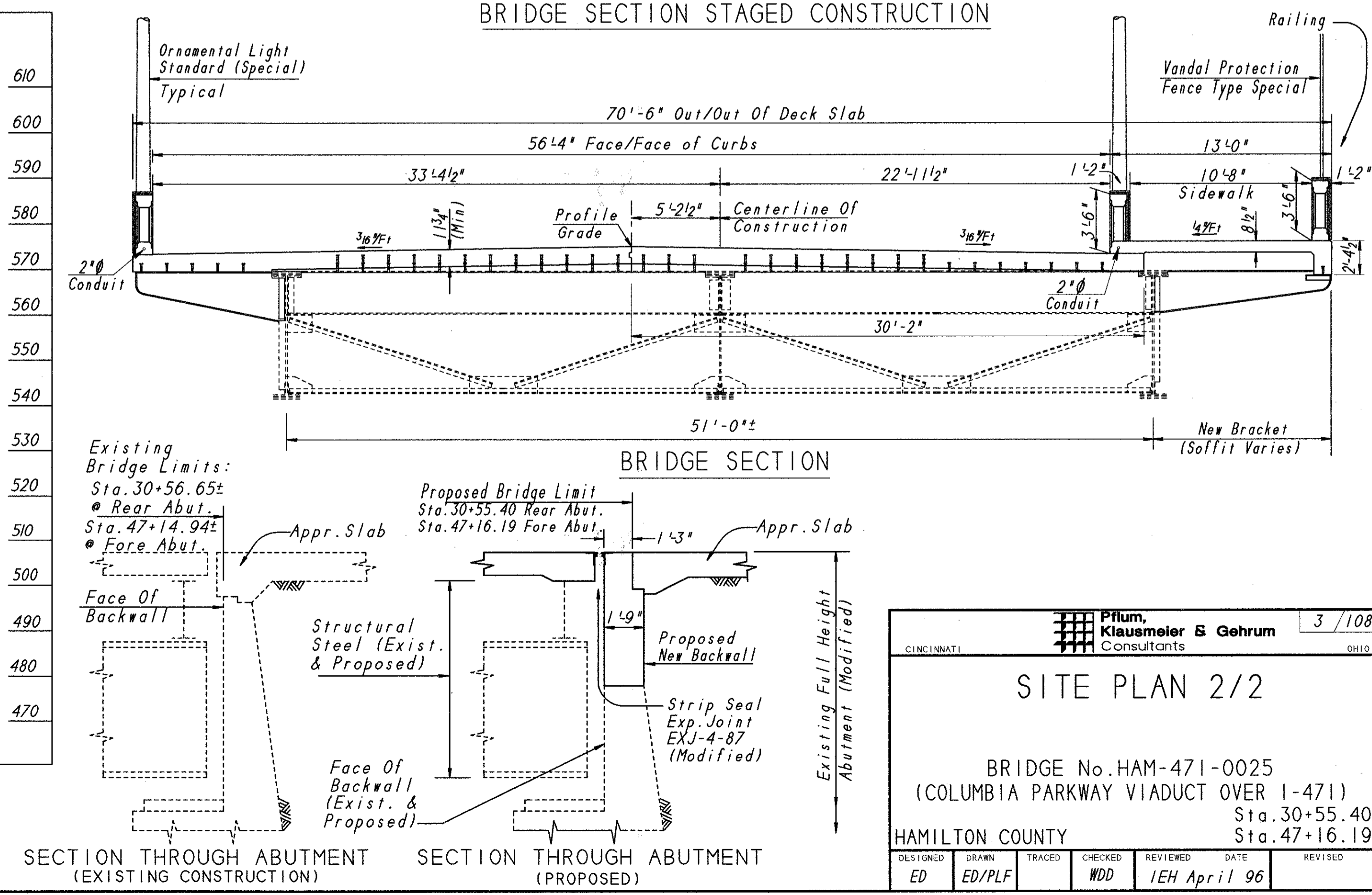
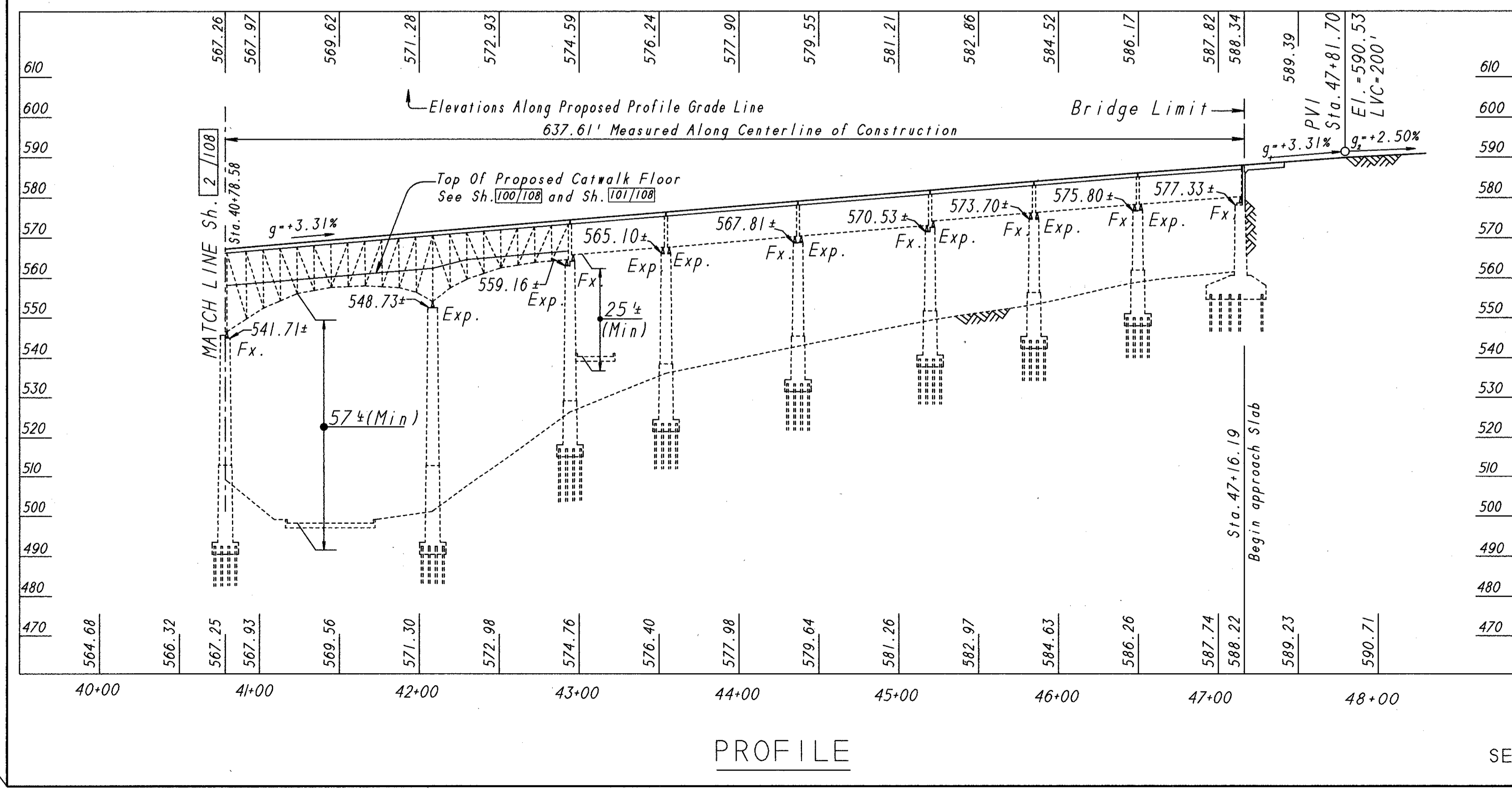
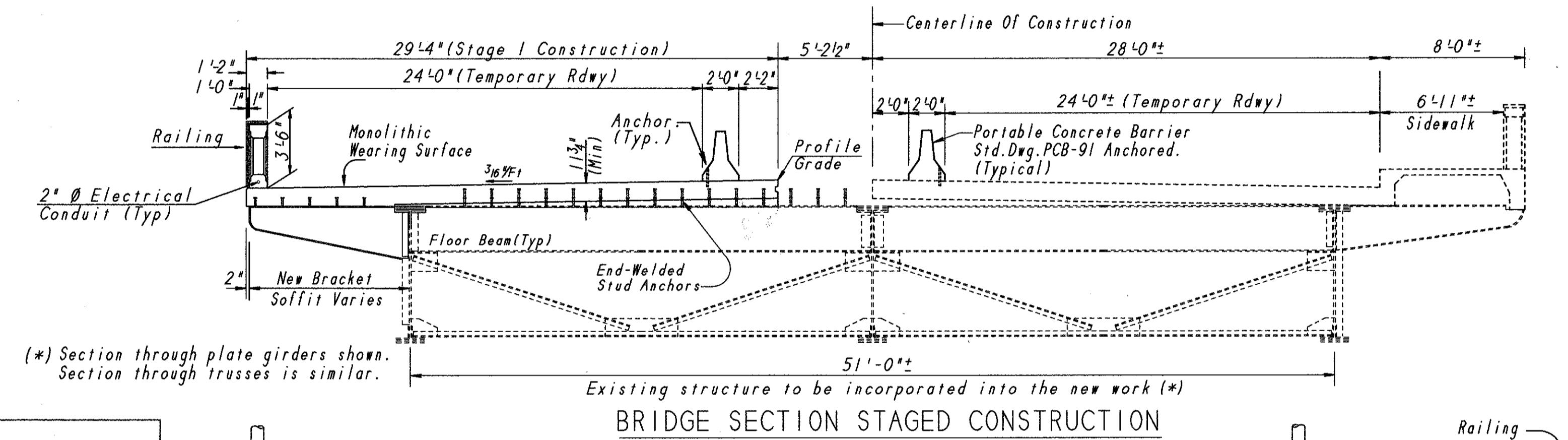
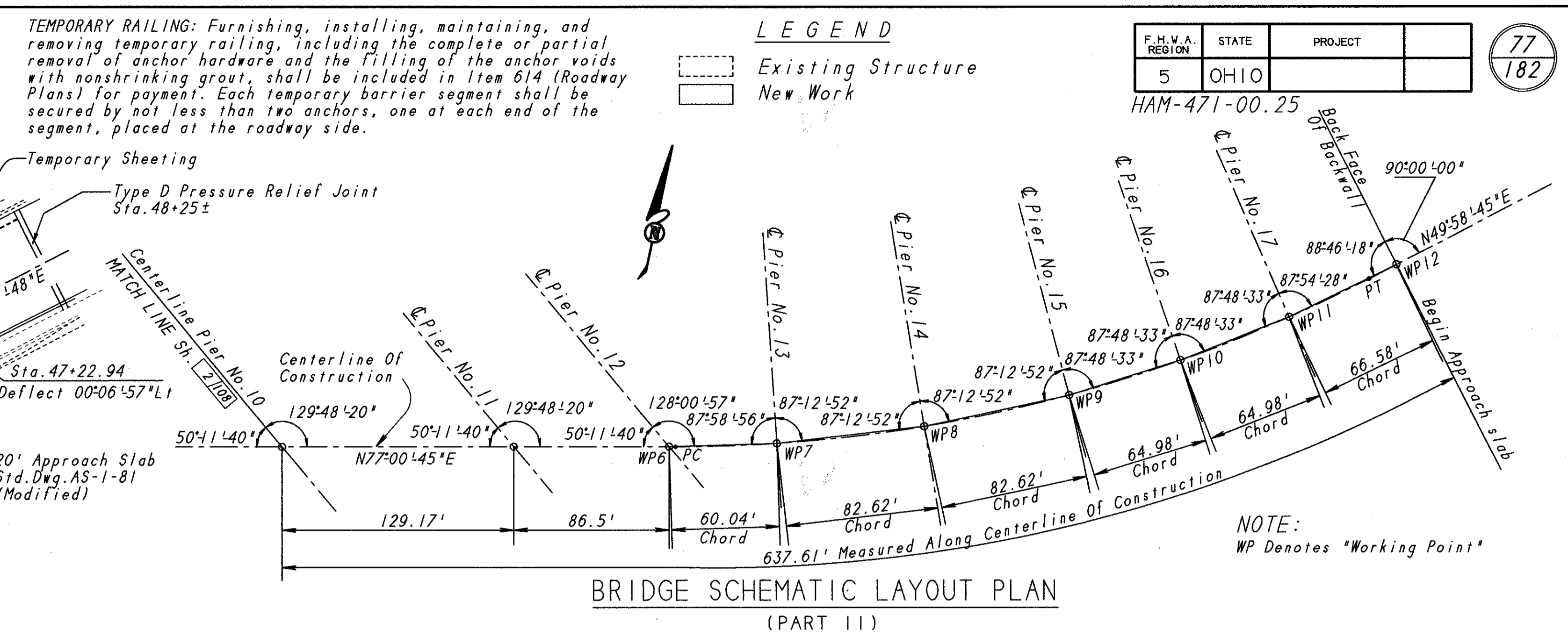
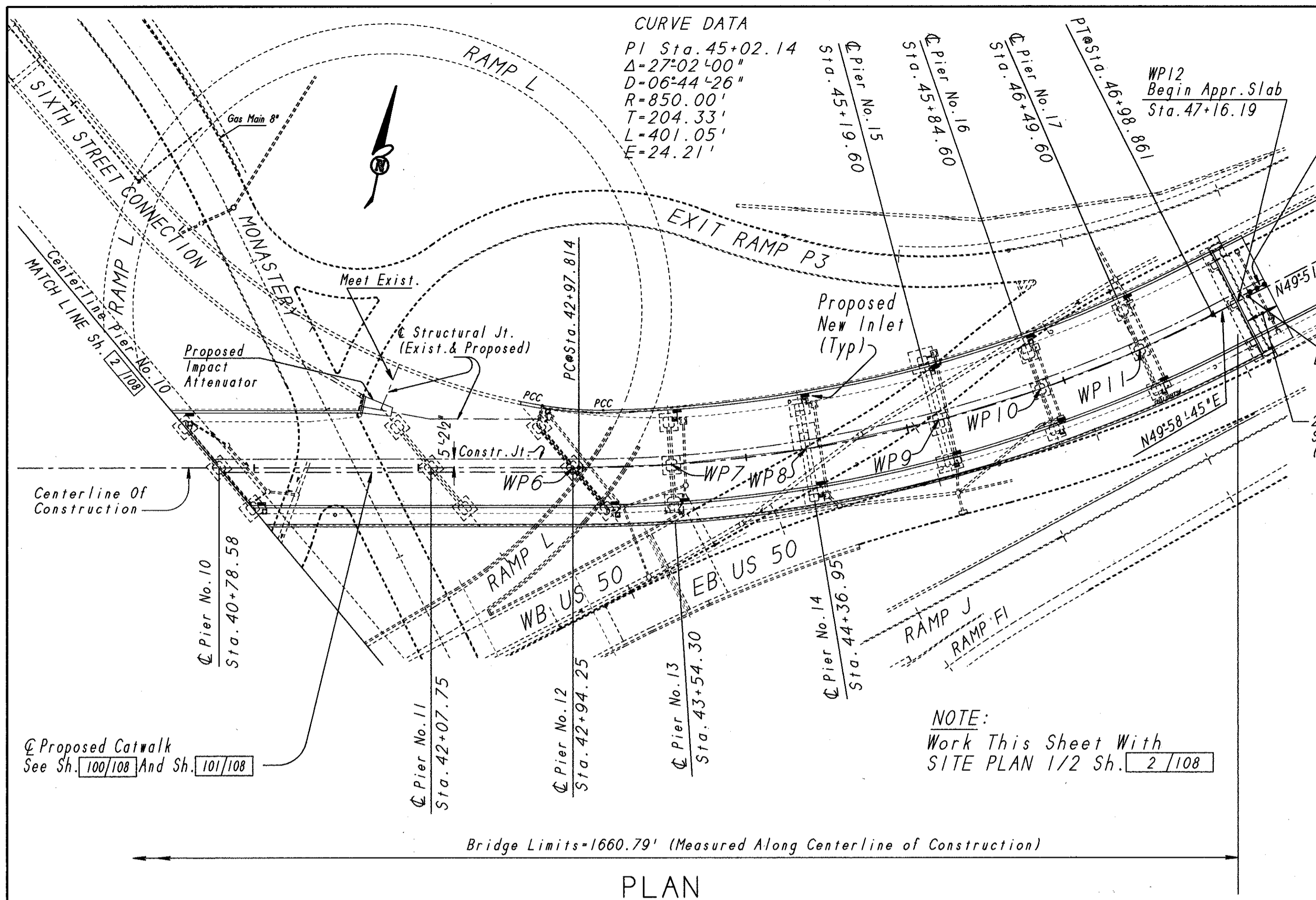
Bridge Limits-1660.79' (Measured Along Centerline of Construction)

CURVE DATA
 PI Sta. 31+93.27
 Δ=25°40'04"
 D=08°11'06"
 R=700.00'
 T=159.47'
 L=313.59'
 E=17.94'

LEGEND
 Existing Structure
 New Work

CURVE B
 PC 30+41.61-34.63' Lt
 PT 31+55.92-33.38' Lt
 Δ = 7°-37'-43"
 R = 900'
 Lc = 119.93'
 Chord = 119.74'

BENCH MARK:
 Chiseled 'X' in concrete at top of steps of existing stairway.
 Sta. 30+33.5, 41.8' Left
 Elev. = 538.30



F.H.V.A. REGION	STATE	PROJECT	77 182
5	OHIO		

HAM-471-00.25

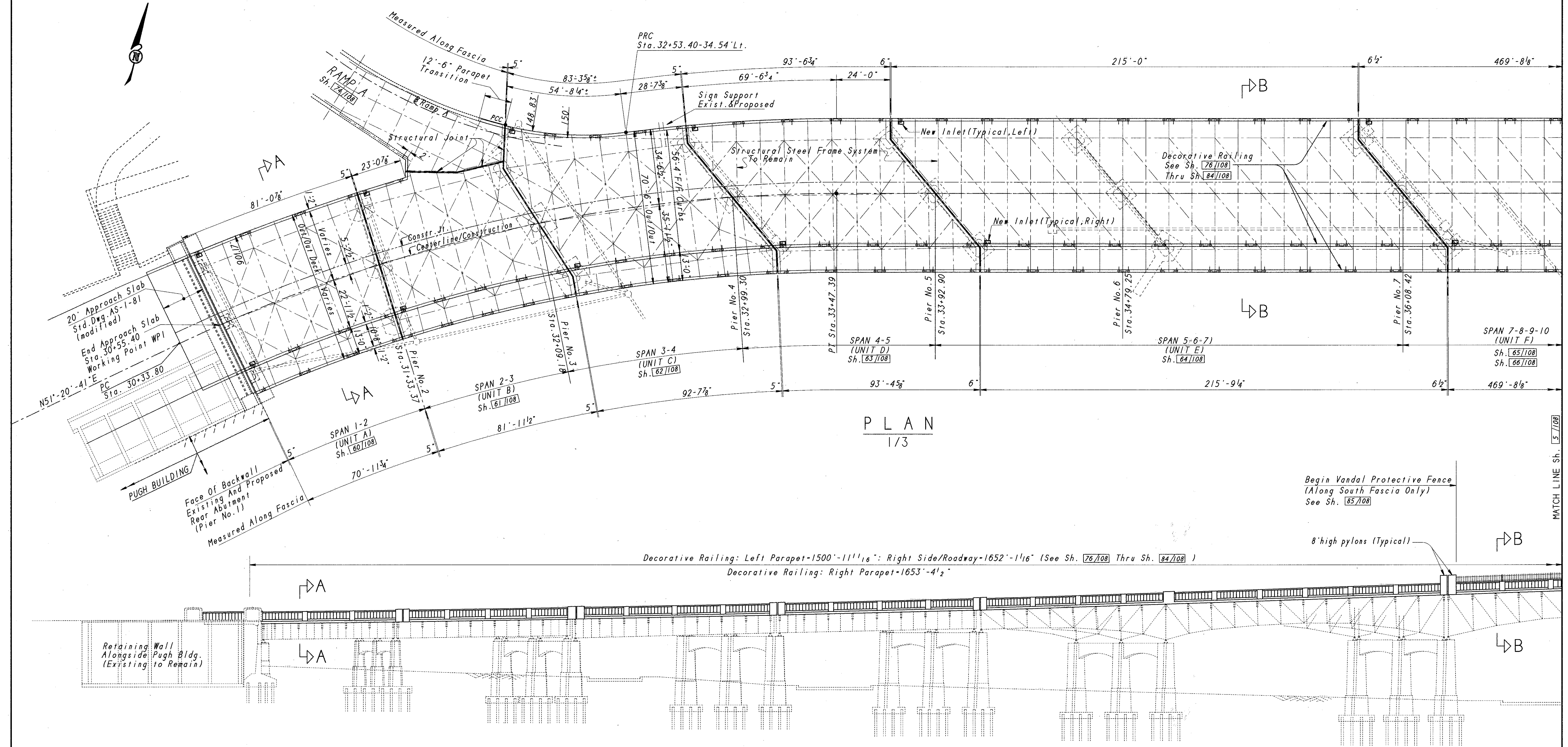
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED/PLF		WDD	IEH	April 96	

Pflum, Klausmeier & Gehrum
 CONSULTANTS
 CINCINNATI, OHIO

SITE PLAN 2/2

BRIDGE No. HAM-471-0025
 (COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
 Sta. 30+55.40
 Sta. 47+16.19



PLAN
1/3

ELEVATION
(LOOKING NORTH)
1/3

- NOTES:
- 1.- Work this sheet with SITE PLAN 1/2 Sh. [2/108] and SITE PLAN 2/2 Sh. [3/108]
 - 2.- Work This Sheet With GENERAL PLAN 2/3 Sh. [5/108] and GENERAL PLAN 3/3 Sh. [6/108]
 - 3.- See GENERAL NOTES, Sh. [7/108] Thru Sh. [11/108]

LEGEND

	Existing Structure
	New Work

Begin Vandal Protective Fence
(Along South Fascia Only)
See Sh. [85/108]

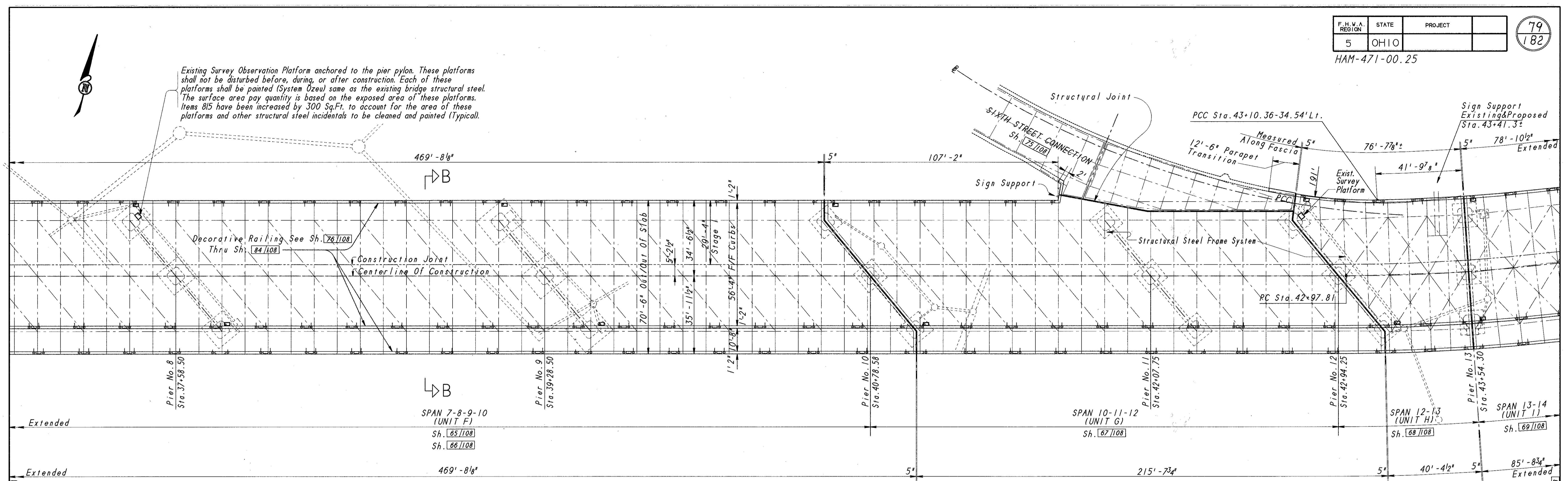
8' high pylons (Typical)

Decorative Railing: Left Parapet-1500'-11 1/16" ; Right Side/Roadway-1652'-1 1/16" (See Sh. [76/108] Thru Sh. [84/108])
Decorative Railing: Right Parapet-1653'-4 1/2"

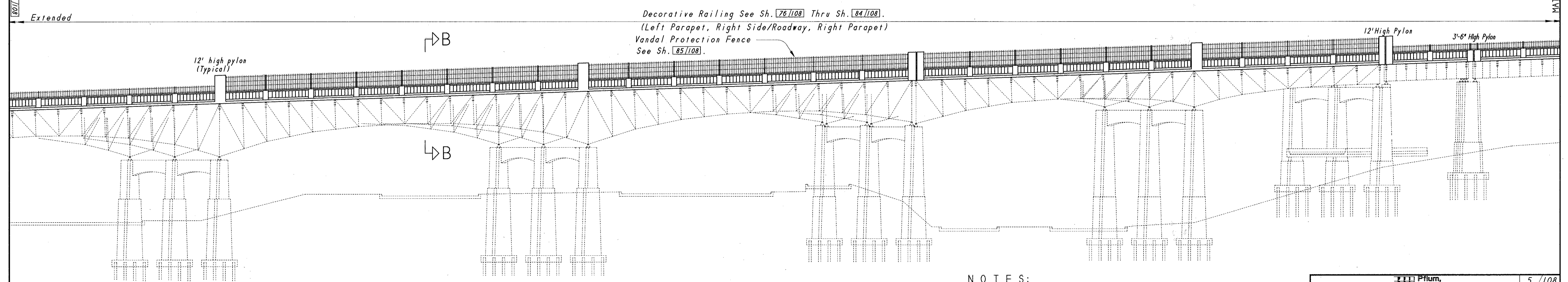
Plum, Klausmeyer & Gehrum Consultants		4 / 108
GENERAL PLAN 1/3		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
		Sta. 30+55.40 Sta. 47+16.19
HAMILTON COUNTY		
DESIGNED	DRAWN	TRACED
ED	ED/PLA	WDD
CHECKED	REVIEWED	DATE
	IEH April 96	

GEN/REV/1 Scale 21:3333

Existing Survey Observation Platform anchored to the pier pylon. These platforms shall not be disturbed before, during, or after construction. Each of these platforms shall be painted (System Ozeu) same as the existing bridge structural steel. The surface area pay quantity is based on the exposed area of these platforms. Items 815 have been increased by 300 Sq.Ft. to account for the area of these platforms and other structural steel incidentals to be cleaned and painted (Typical).



PLAN
2/3



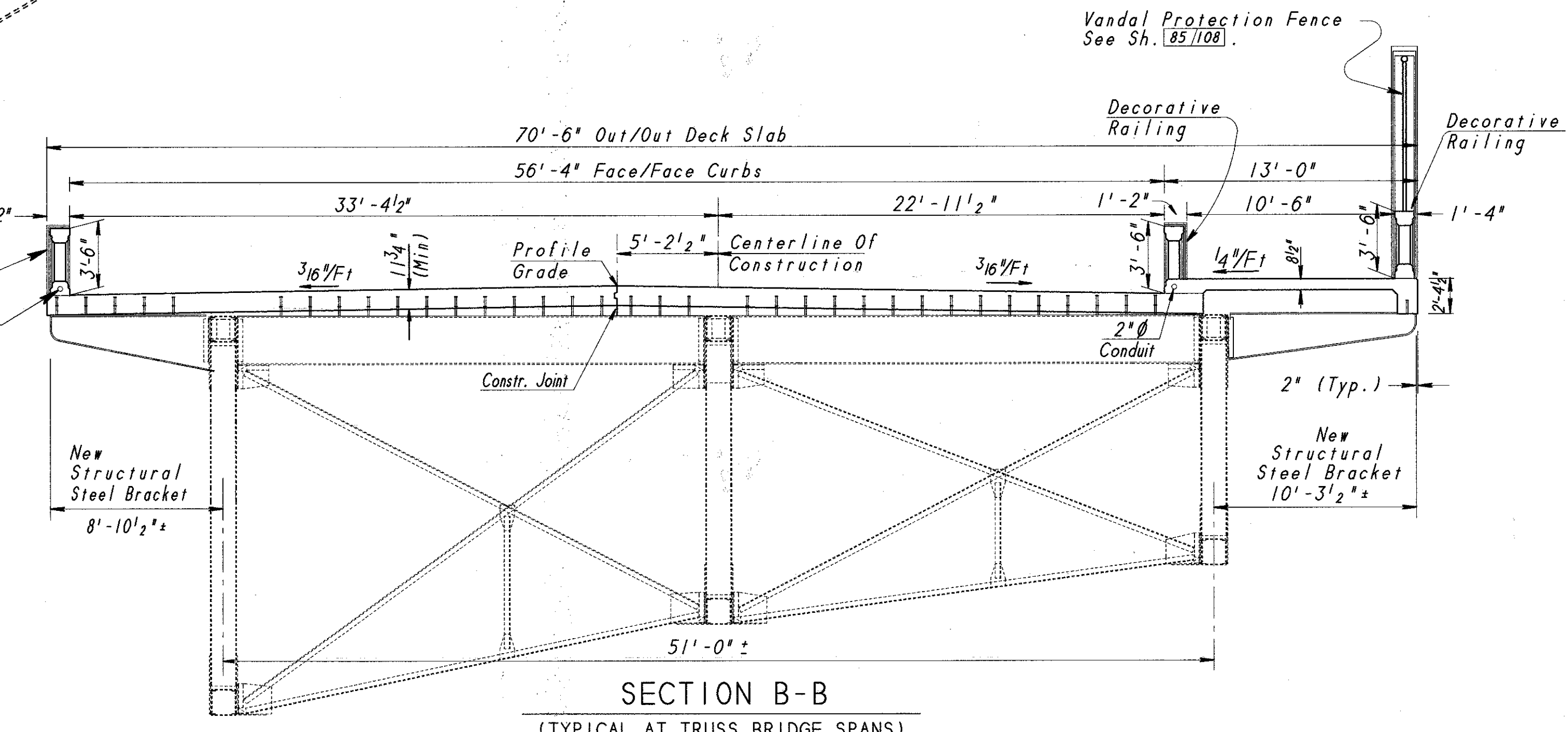
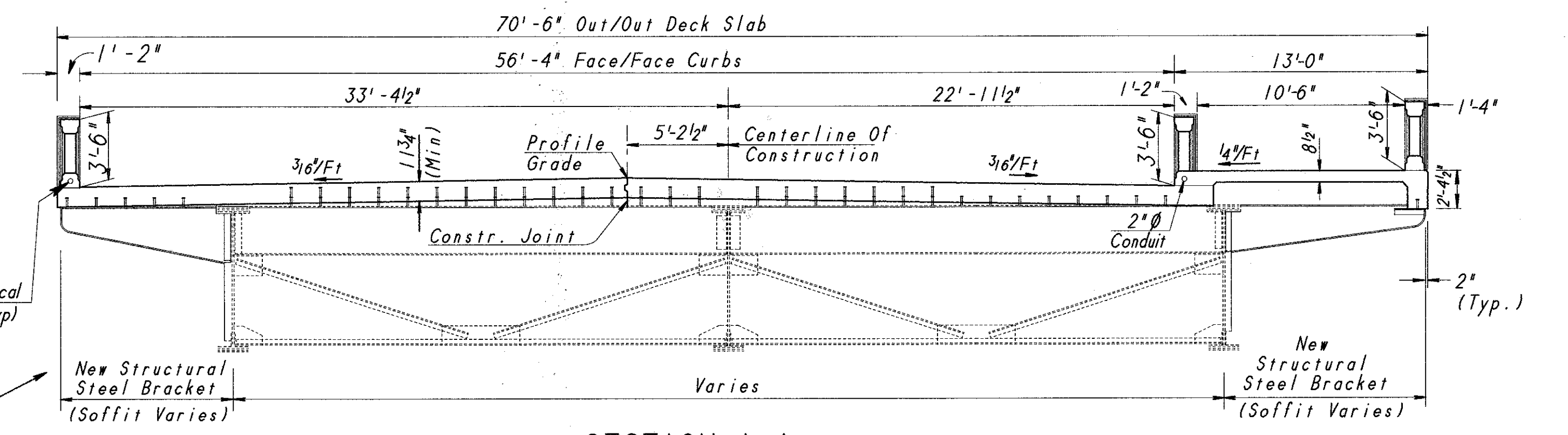
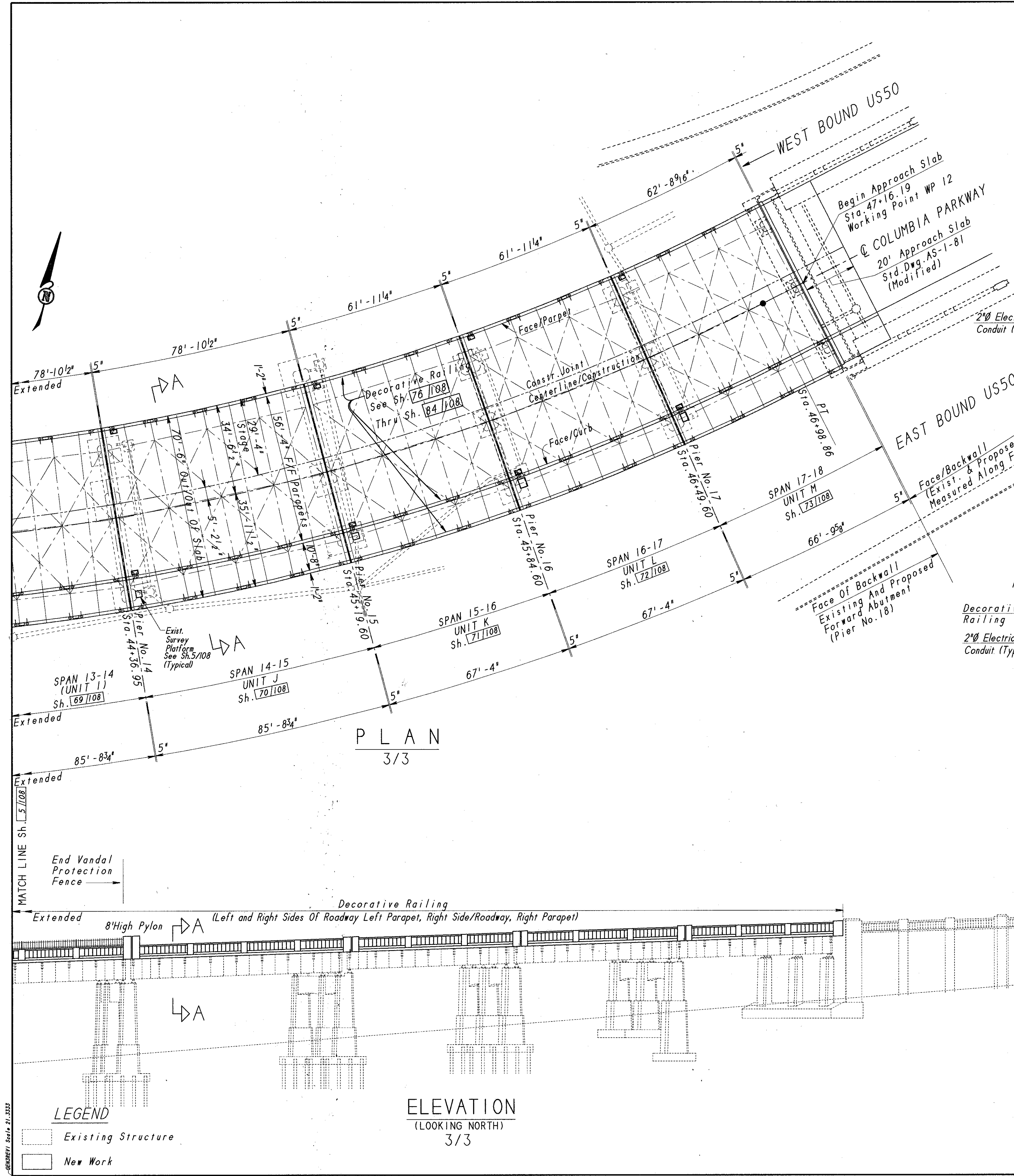
ELEVATION
(LOOKING NORTH)
2/3

LEGEND
 Existing Structure
 New Work

- NOTES:**
- 1.-Work this sheet with **SITE PLAN 1/2**, Sh. [2/108] and **SITE PLAN 2/2**, Sh. [3/108]
 - 2.-Work this sheet with **GENERAL PLAN 1/3**, Sh. [4/108] and **GENERAL PLAN 3/3**, Sh. [6/108]
 - 3.-See **GENERAL NOTES**, Sh. [7/108] Thru Sh. [11/108].

Plum, Klausmeyer & Gehrum Consultants		5/108
GENERAL PLAN 2/3		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

GENEVEY Sept. 21, 1993



- NOTES:**
1. - Work this sheet with SITE PLAN 1/2, Sh. 2/108 and SITE PLAN 2/2, Sh. 3/108
 2. - Work This Sheet With GENERAL PLAN 1/3, Sh. 4/108 and GENERAL PLAN 2/3, Sh. 5/108
 3. - See GENERAL NOTES, Sh. 7/108 Thru Sh. 11/108.

Plum, Klausmeier & Gehrm Consultants		6/108
GENERAL PLAN 3/3		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471) Sta. 30+55.40 Sta. 47+16.19		
HAMILTON COUNTY		
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96



ESTIMATED QUANTITIES

Calc.: WDD: Date: Sept. 95
Chk.: ED: Date: Apr. 96

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	SUP'R	ABUTS	PIERS	GEN'L
202	11203	Lump	Sum	Portions Of Structures Removed, over 20 foot Span, As Per Plan.				Lump
202	75402	20	Each	Light Pole Removed For Storage	20			
202	75504	20	Each	Luminaire Removed For Storage.	20			
503	11100	Lump	Sum	Cofferdams, Cribbs And Sheeting				Lump
503	21300	Lump	Sum	Unclassified Excavation				Lump
509	15840	626686	Pound	Epoxy Coated Reinforcing Steel, Grade 60	617631	8255		800
510	09950	81	Each	Dowel Holes With Cement Grout		27		54
511	31502	26	Cu.Yd.	Class S Concrete, Superstructure,	26			
511	45700	64	Cu.Yd.	Class C Concrete, Abutments, Repair or Reconstruction.		64		
511	71100	4584	Cu.Yd.	Concrete Misc.: Lightweight Concrete Superstructure, As Per Plan.	4584			
512	44400	50	Sq.Yd.	Type B Waterproofing		50		
Special	51267502	6138	Sq.Yd.	Sealing Of Concrete Surfaces (Epoxy) *		618	5520.	
Special	51267504	8224	Sq.Yd.	Sealing Of Concrete Surfaces (Non-epoxy)*	8224			
513	11700	329375	Pound	Structural Steel, AISC Category III.	329375			
513	15901	65301	Pound	Structural Steel, Replacement Of Deteriorated End Cross Frames, As Per Plan.	65301			
513	16590	101479	Pound	Structural Steel, Misc: Catwalk Including Ladders.				101479
513	17502	2337	Sq.Ft.	Catwalk Grating				2337
513	20000	22054	Each	Welded Stud Shear Connector	22054			
Special	51400620	38794	Sq.Ft.	Field Painting Of New Steel, System IZEU *	38794			
516	11210	1244	Lin.Ft.	Structural Expansion Joint Including Elastomeric Strip Seal. *	1244			
516	44401	1	Each	Elastomeric Bearing With Internal Laminates And Load Plate (Neoprene), As Per Plan (2'-2 1/2" x 2'-1" x 6 1/8" Pad & 2' 6 1/2" x 2'-2" x 3" Plate)				
516	44401	29	Each	Elastomeric Bearing With Internal Laminates And Load Plate (Neoprene), As Per Plan (2'-2 1/2" x 2'-1" x 6 1/8" Pad & 2' 3 1/2" x 2'-2" x 3" Plate)				
516	45001	6	Each	Steel Pot Bearing, As Per Plan, 300 kips *	6			
516	45001	6	Each	Steel Pot Bearing, As Per Plan, 500 kips *	6			
516	45001	6	Each	Steel Pot Bearing, As Per Plan, 1000 kips *	6			
516	45001	3	Each	Steel Pot Bearing, As Per Plan, 1200 kips *	3			
Special	51645100.	3	Each	Bearing Test, Steel Pot Bearing	3			
Special	51645300	3	Each	Additional Bearing Test, Steel Pot Bearing	3			
516	47001	Lump	Sum	Jacking And Temporary Support Of Superstructure As Per Plan.	Lump			
517	74500	4807	Lin.Ft.	Railing, Concrete (Decorative Type).	4807			
518	21200	63	Cu.Yd.	Porous Backfill With Filter Fabric		63		
518	51100	1250	Lin.Ft.	8" Pipe Downspout Including Specials.*	1250			
518	62200	28	Each	Structure Drainage, Misc.: Inlet, Including Steel Support Beams, Frame And Specials, 8" Downspout And Grating. *	28			
518	63300	Lump	Sum	Structure Drainage, Misc.: Straightening, Adjusting And Cleaning Existing 6" Downspout Including Specials.				Lump
Special	51911502	1312	Sq.Ft.	Patching Concrete Structure With Trowelable Mortar *			1312	
Special	51912610	62	Lin.Ft.	Concrete Repair By Epoxy Injection including Surface Preparation.*			62	
Special	51922100	15	Cu.Yd.	Micro-Silica Modified Concrete Overlay (Variable Thickness)*	15			
Special	53000200	Lump	Sum	Structure Misc.: Bridge Identification Sign				Lump
Special	60740000	268	Lin.Ft.	Vandal Protection Fence (4 Ft. High)	268			
Special	60740000	521	Lin.Ft.	Vandal Protection Fence (8 Ft. High)	521			

* See Proposal Note.

ESTIMATED QUANTITIES

Calc.: WDD: Date: Sept. 95
Chk.: ED: Date: Apr. 96

ITEM	ITEM EXT.	TOTAL	UNIT	DESCRIPTION	SUP'R	ABUTS	PIERS	GEN'L
815	00050	282000	Sq.Ft.	Surface Preparation Of Existing Steel, System OZEU	282000			
815	00056	282000	Sq.Ft.	Field Painting Of Existing Steel Prime Coat, System OZEU	282000			
815	00060	282000	Sq.Ft.	Field Painting Of Existing Steel Intermediate Coat, System OZEU	282000			
815	00066	282000	Sq.Ft.	Field Painting Of Existing Steel Finish Coat, System OZEU	282000			
815	00500	103700	Lin.Ft.	Caulking	103700			

GENERAL NOTES

REFERENCE shall be made to Standard Drawings:

AS-1-81 9-15-94
EXJ-4-87 11-12-93
PCB-91 4-24-92

And Supplemental Specifications:

No.	Title	Date
815	Field Painting Of Existing Steel, System OZEU	7-17-95
910	OZEU Structural Steel Paint	7-17-95
933	Quick Setting Concrete Mortar	7-17-95

DESIGN SPECIFICATIONS:

This structure conforms to the "Standard Specifications For Highway Bridges" Fifteenth Edition 1992 adopted by the American Association Of State Highway and Transportation Officials.

DESIGN DATA:

DESIGN LOADING:
HS20-44, Case II and the Alternate Military Loading.

DESIGN STRESSES:
Concrete Class C - Unit Stress 1,333 psi., Abutments.
Concrete Class S - Unit Stress 1,500 psi., Superstructure
Lightweight Concrete - Unit Stress 1,500 psi., Superstructure.

Reinforcing Steel - ASTM A615, A616, A617, Grade 60.
Unit Stress 24,000 psi.

Structural Steel-ASTM A709 Grade 36, Unit Stress 20,000 psi., Catwalk And Replacement Of Deteriorated End Cross Frames.
Structural Steel-ASTM A709 Grade 50, Unit Stress 27,000 psi., New Brackets.

Cast Steel For Steel Castings-ASTM A27 Grade 36.

High Strength Bolts ASTM A325

UTILITY LINES:

All expense involved in relocating (installing) the affected utility lines shall be borne by the Utility(ies). The Contractor and Utility(ies) are to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

DECK PROTECTION METHOD: Epoxy Coated Reinforcing Steel and 2-1/2 in. cover.

MONOLITHIC WEARING SURFACE is assumed, for design purposes, to be 1 in. thick.

ALL SURFACES to be patched and the exposed reinforcing steel within shall be thoroughly cleaned by abrasive blasting prior to the cleaning specified by 520.05. Cleaning shall precede application of the patching material or erection of the forms by not more than 24 hours.

REINFORCING STEEL: New reinforcing steel may require field cutting or bending to be properly fitted. Payment shall be included in 509.

MAINTENANCE OF TRAFFIC:

Two lanes of traffic within a temporary roadway of 24 feet minimum width shall be maintained at all times as described on the Roadway Plan.

EXISTING STRUCTURE VERIFICATION:

Details and dimensions shown on these plans pertaining to the existing structure have been obtained from plans of the existing structure and/or field observations and measurements. Consequently, they are indicative of the existing structure and the proposed work but they shall be considered tentative and approximate. The Contractor is referred to CMS Sections 102.05, 105.02, and 513.02.

Contract bid prices shall be based on a recognition of the uncertainties described above and on a prebid examination of the existing structure by the Contractor. However, all project work shall be based on actual details and dimensions which have been verified by the Contractor in the field.

EXISTING STRUCTURE PLANS. -

Existing structure plans may be inspected in the office of the City Engineer, City Hall, Room 445, 801 Plum Street Cincinnati, Ohio.

PROPOSED WORK. -

The major portions of this work include:

1. - Jack Superstructure
2. - Remove existing bearing devices and replace with new at locations designated in the plans.
3. - Remove designated end cross frames of the existing structural steel superstructure and replace with new structural steel of the same configuration.
4. - Remove existing concrete deck, concrete railing and any appurtenances related to the concrete deck.
5. - Remove abutment backwalls, approach slabs, and portions of abutment pylons.
6. - Remove designated brackets of the existing structural steel superstructure and replace with new brackets.
7. - Construct new abutment backwalls, approach slab seat and abutment pylons.
8. - Construct new concrete deck and decorative concrete railing, pedestrian protection fence, and other appurtenances related to the deck.
9. - Patch designated concrete surfaces of the piers.
10. - Install steel catwalk.
11. - Painting of existing structural steel structure to remain.
12. - Painting of new structural steel.
13. - Disposal of removed material away from the bridge site.

STRUCTURE EXCAVATION:

Structure excavation in addition to that necessary to remove portions of the existing structure, and all necessary backfill are included in the lump sum price bid with Item 503 UNCLASSIFIED EXCAVATION for payment.

CINCINNATI		Plum, Klausmeyer & Gehrmann Consultants		7/108
ESTIMATED QUANTITIES AND GENERAL NOTES 1/5				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
				Sta. 30+55.40
				Sta. 47+16.19
HAMILTON COUNTY				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
ED	ED/PLF		WDD	IEH April 96

ITEM 516 - STEEL POT BEARINGS, AS PER PLAN TYPE GUIDED

DESCRIPTION -

This item shall consist of furnishing all materials, labor, tools, equipment and incidentals necessary to design, fabricate, test and install pot bearings in accordance with the plans and these specifications. The basic configuration for the bearings shall meet the requirements of these specifications and as shown on Sh. [36/108] thru Sh. [37/108].

1. THE POT BEARING SHALL CONSIST OF THE FOLLOWING PARTS:

1.1) RECTANGULAR SOLE PLATE. - Top side flat and field welded to the existing cast steel shoe that has been modified with an additional plate as shown on Sh. [36/108]. The sole plate to be added to the existing cast steel shoe shall have both top and bottom faces with flatness Class A tolerance and the face to be welded to the cast steel shoe shall be provided with a groove machined to accept the underside of the shoe. The top plate of the pot bearing shall have integral guide bars. The sliding surface of the plate and guide bars shall be provided with stainless steel sheets. Top plate rides on piston.

1.2) CIRCULAR PISTON. - Faced with Polytetrafluoroethylene (PTFE). Piston sits in steel pot on lubrication and elastomeric discs.

1.3) ELASTOMERIC DISC. - Confined within pot for the purpose of providing rotation and support for the piston. Lubricating compound is to be provided above and below the elastomeric disc. The disc shall be sealed with brass sealing rings.

1.4) SEALING RINGS. - Seal between pot and piston to contain the elastomeric disc.

1.5) GUIDE BARS. - Attached or integral with the bearing top plate to guide bearing and transmit lateral loads to the pot.

1.6) CIRCULAR POT. - Containment for the elastomeric disc and transmission of vertical and lateral loads to masonry plate. Field Welded To Masonry Plate.

1.7) MASONRY PLATE. - Distribute vertical and horizontal forces from the steel pot to the concrete bridge seat. Masonry plate sits on bearing pad and is connected to the concrete with anchor bolts.

2. DESIGN AND MATERIALS REQUIREMENTS:

Schematic drawings indicating basic concepts of pot bearings are shown in the project plans for information only. The bearings shall be designed to be removable and replaceable with a jacking movement not to exceed 1/4 inch. The manufacturer may modify details of his design to meet specific requirements subject to review and approval of the Engineer. The design requirements for load, movements, and rotations are shown in the project plans.

The bearing shall be designed by Service Load Design Method of the AASHTO Standard Specifications For Highway Bridges, 15th Edition. Seismic performance category "B" and acceleration coefficient of 0.10 shall be used for seismic design.

Stresses in the steel plates of the bearings and the stresses imposed on the adjacent concrete surfaces shall be within the allowable stress values of the AASHTO Standard Specifications for Highway Bridges, 15th Edition.

2.1) BEARING HEIGHT. -

The total bearing height shall meet the existing clear distance shown in Table B, Sh. [36/108]. This dimension shall be met by increasing the sole plate thickness or pot bearing sliding plate thickness or the masonry plate thickness or a combination thereof.

2.2) SOLE PLATE WELDED TO THE EXISTING CAST STEEL SHOE. -

ASTM A709 Grade 50 or Grade 50W steel plate.

2.3) BEARING SOLE PLATE. -

(2.3.1) ASTM A709 Grade 50 or Grade 50W steel with stainless steel sliding surface on under side. Top side flat to match the plate described on 2.2 above. Bottom side level.

(2.3.2) Stainless steel sheet surface shall conform to ASTM A167 or A240 Type 304. The minimum thickness shall be 0.06 inch. Stainless steel in contact with PTFE shall have a 20 micro-inch RMS finish or better. The surface shall be mechanically polished. Material and finish shall be such that the requirements of friction test as described in these specifications are met.

(2.3.3) The plate shall be Rectangular or square in plan.

(2.3.4) Plan dimensions shall accommodate the pot diameter, guide bars, design movement as shown on Sh. [36/108] plus 0.25 inch.

(2.3.5) Minimum thickness shall be 0.75 inches.

(2.3.6) Expansion guide bars as described in section 2.7 of these Specifications.

2.4) PISTON. -

(2.4.1) ASTM A709 Grade 50 or Grade 50W steel.

(2.4.2) Diameter of piston shall be 0.03 to 0.05 inch less than the inside diameter of the pot.

(2.4.3) Piston thickness shall be sufficient to provide a clearance of (0.02xPot O.D.+0.12) inches between the top of the pot wall and surface above (guide bar or bottom of sole plate) the pot wall when the piston is in an unrotated position. The piston thickness shall be sufficient to transmit two times the lateral loads shown on Sh. [36/108] from the guide bar to the pot wall without deflection or distortion.

(2.4.4) Piston walls shall be tapered inward, toward the top, to prevent binding against the pot walls during rotation, and the bottom edge shall be rounded with a machined 0.125 inch radius.

(2.4.5) The piston shall be machined from a single piece of structural steel.

(2.4.6) The thickness of the piston shall ensure that the bottom of the piston will be entirely below the top of the pot up to 200 percent of maximum design rotation.

(2.4.7) The top of the piston and the sides of guide bar recess shall be faced with PTFE. The PTFE surface shall consist of finished unfilled PTFE sheet made from virgin PTFE resin OR 100 percent PTFE fabric made from virgin PTFE multifilament fiber. material and finish shall be such that the requirements of Section 2.4.8 are met.

(2.4.8) PTFE fabric fibers shall conform to the following:

- A) The resin from which the fibers are produced shall be 100 percent PTFE conforming to ASTM D1457.
- B) Tensile Strength ASTM D2256, 24,000 psi (Minimum).
- C) Elongation ASTM D2256, 75% (Minimum).
- D) The PTFE fabric shall have a minimum thickness of 0.0625 inch (compressed). Maximum thickness shall be 0.125 inch (compressed).

(2.4.9) Finished unfilled PTFE sheet shall be made from 100 percent virgin PTFE resin and shall conform to the following requirements:

- A) Tensile strength ASTM D1457, 2800 psi (Minimum).
- B) Elongation ASTM D1457, 200 percent (Minimum).
- C) Specific Gravity ASTM D1457, 2.13 (Minimum).
- D) Melting point ASTM D1457, 327° Centigrade ±10.
- E) Minimum thickness shall be 0.187 inch. Sheet shall be recessed one half its thickness into steel substrata.
- F) PTFE sheet shall be commercially etched on its bonding side.

2.5) ELASTOMERIC DISC. -

(2.5.1) The elastomeric disc shall meet the following average compressive stress requirements:

- (A) Maximum of 3500 psi when the bearing vertical design capacity specified in the plan is applied to the area of the disc.
- (B) Minimum of 700 psi when the lesser of the dead load or 20% of the bearing vertical design capacity specified in the plan is applied to the area of the disc.

(2.5.2) Minimum disc thickness shall be 0.067 times Disc Diameter.

(2.5.3) The elastomeric disc shall consist of 100 percent virgin Polychloroprene (Neoprene) meeting the requirements of CMS 711.23 or 100 percent virgin natural Polyisoprene (Natural Rubber) meeting the requirements of the current AASHTO M251.

(2.5.4) Hardness shall be 50 Durometer +/- 5

(2.5.5) The disc shall consist of one solid piece of elastomer.

(2.5.6) The elastomeric disc shall be lubricated with a silicone compound conforming to the requirements of MIL-S-8660 or approved equal.

(2.5.7) Two flat brass sealing rings shall be used to seal the disc. The upper edge of the disc shall be recessed to receive the sealing rings so that they sit flush with the upper surface of the elastomeric disc.

2.6) SEALING RING. -

(2.6.1) Rings shall be flat and made of brass conforming to the requirements of ASTM B36, half hard.

(2.6.2) Minimum width shall be 0.375 inch.

(2.6.3) Minimum thickness shall be 0.050 inch.

(2.6.4) The rings shall have a smooth finish of 64 micro-inch (RMS) or less.

(2.6.5) Two rings are required.

(2.6.6) The rings shall be split and snugly fit the recess in the elastomeric disc as well as the inside diameter of the pot. The ends of the rings at the split shall be cut at 45 degrees to the vertical. The maximum gap shall be 0.050 inch when installed. The rings shall be arranged to have the splits staggered a minimum of 90 degrees relative to one another.

2.7) GUIDE BARS. -

(2.7.1) ASTM A709 Grade 50 or A709 grade 50W faced with stainless steel.

(2.7.2) Guide bars may be integral by machining from a solid sole plate or they may be attached to the sole plate by press fit into recess and welding the ends. The side surfaces of the guide bars shall be faced with stainless steel (Section 2.3.2 of these specifications). Welding of guide bars to the sole plate shall be performed prior to welding of stainless steel to the sole plate or guide bar.

(2.7.3) The total space (both sides) between the guide bars and guided members shall be 0.125 minus 0, plus 0.0625 inch.

(2.7.4) The guide bars shall be designed for no less than two times the lateral horizontal force shown on Sh. [36/108].

(2.7.5) Guiding arrangements shall be designed so that the guided member is always within the guides at all points of translation and rotation of the bearing.

(2.7.6) The bottom of the recess shall be a minimum of 0.375 inch clear to guide bar in sole plate.

2.8) POT

(2.8.1) ASTM A709 Grade 50 or A709 Grade 50W Structural Steel.

(2.8.2) The pot shall consist of a solid plate into which a circular recess has been machined.

(2.8.3) The depth of the circular recess shall be equal to, or greater than (Pot I.D. times 0.02 plus 0.10 inches plus thickness of the elastomeric discs).

(2.8.4) The pot inside diameter shall be the same as the elastomeric disc. (See Section 2.5).

(2.8.5) The outside of the pot shall be rectangular.

(2.8.6) The thickness of the pot wall shall be sufficient to transmit two times the lateral horizontal force shown on Sh. [36/108] to the pot base with the load applied at a point contact at two times the design rotation and accounting for internal elastomer pressure without causing deflection or distortion to the pot wall or base. In no case shall the wall thickness be less than 3/4 inches.

(2.8.7) The minimum thickness of the pot beneath the elastomer for a bearing directly on a masonry plate shall be not less than the greater of 0.045 times Pot I.D. or 0.50 inch and meet the requirements (2.8.6) above.

(Continued Next Page)

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GENERAL NOTES 2/5				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
				Sta. 30+55.40
				Sta. 47+16.19
HAMILTON COUNTY				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
ED	ED		WDD	IEH April 96

ITEM 516 - STEEL POT BEARING (Cont..)

2.9) MAKEUP PLATE.-

(2.9.1) ASTM A709 Grade 50 or A709 Grade 50W Structural Steel.

2.10) MASONRY PLATE.-

(2.10.1) ASTM A709 Grade 50 or A709 Grade 50W Structural Steel.

2.11) BEARING PAD SHEET LEAD.-

(2.11.1) Bearing pad sheet lead shall conform to ASTM B-29. Preformed bearing pads shall conform to CMS 711.21.

3.- FABRICATION.-

3.1) ATTACHMENT OF SHEET PTFE TO SUBSTRATE.-

(3.1.1) PTFE sheet shall be recessed into, and bonded to a steel substrate.

(3.1.2) PTFE shall be recessed for one half its thickness.

(3.1.3) The bonding surface of the steel shall be cleaned of rust, scale, oil and grease by blast cleaning and then wiped clean with a cleaning solvent. Blast cleaning shall be performed within a maximum of four hours prior to bonding.

(3.1.4) The adhesive material and the bonding procedures to be used shall be submitted to the Director for approval prior to performance of the bonding operation. The bonding operation shall then be performed under controlled conditions and in accordance with these approved procedures.

(3.1.5) After completion of the bonding operation, the PTFE surface shall be smooth and free from bubbles.

3.2) ATTACHMENT OF PTFE FABRIC TO SUBSTRATE.

(3.2.1) PTFE fabric shall be mechanically interlocked and bonded to the steel substrate.

(3.2.2) The bonding surface of the steel shall be cleaned of rust, scale, oil and grease by blast cleaning and then cleaned with solvent. Blast cleaning shall be performed within a maximum of four hours prior to bonding.

(3.2.3) The mechanical interlock and adhesive bonding material and procedures shall be submitted to the Director for approval prior to performance of the bonding operation. The bonding operation shall then be performed under controlled conditions and in accordance with these approved procedures as approved by the Director.

(3.2.4) Migration of epoxy through the fabric will not be permitted.

(3.2.5) Fabric shall be furnished in one piece. Edges shall be oversewn or recessed so that no cut fabric edges are exposed.

3.3) ATTACHMENT OF SHEET STAINLESS STEEL.

(3.3.1) Stainless steel shall be attached to its steel substrate with an approved epoxy to ensure complete contact, and then seal welded. Seal welds shall be continuous for the entire periphery of the stainless steel overlay. The entire stainless steel surface shall conform to the requirements of section (2.3.2) after welding.

3.4) CORROSION PROTECTION.-

(3.4.1) All steel surfaces (including A709 Grade 50 Steel) exposed to the atmosphere, except stainless steel surfaces, shall be shop prime coated System IZEU.

3.5) WELDING.-

(3.5.1) Welding as a means of attachment shall be done in a controlled manner and shall conform to CMS 513. Welding to a steel plate which has bonded PTFE surface may be permitted providing welding procedures are established which restrict the maximum temperature reached by the bond area to less than 300 degrees Fahrenheit, as determined by temperature indicating pencils, or other suitable means.

3.6) TOLERANCES.-

(3.6.1) General Flatness Criteria:

A.- Flatness tolerances shall be defined as:

A1).- Class A Tolerance = 0.0005 x Nominal Dimension.

A2).- Class B Tolerance = 0.0010 x Nominal Dimension.

A3).- Class C Tolerance = 0.0020 x Nominal Dimension.

A4).- Nominal Dimension shall be defined as the actual dimension of the plate, in inches, spanned by the straightedge.

B.- Flatness shall be determined by placing a straightedge, longer than the nominal dimension to be measured, in contact with the surface to be measured or as parallel to it as possible. Select a feeler gauge having a tolerance of plus or minus 0.001 inch and attempt to insert it under the straightedge. (The smallest number of blades shall be used). Flatness is acceptable if the feeler does not pass under the straightedge. The straightedge may be located at any position on the surface and not necessarily at 90 degrees to the edges.

C.- TOLERANCES - SOLE PLATE.-

C1).- Plan dimensions over 30 inches: -0", +1/4"

C2).- Plan dimensions under 30 inches: -0", +3/16"

C3).- Flatness of surface in contact with beam or girder: Class B.

C4).- Flatness of backing surface for stainless steel: Class A

C5).- Thickness: -1/32", +1/8"

D.- TOLERANCES - PISTON.-

D1).- Diameters greater than 20 inches: +0.007"

D2).- Diameters less than 20 inches: +0.005"

D3).- Expansion bearings where upper side is faced with PTFE, flatness of upper side shall be class A.

D4).- Flatness of lower side: Class B.

E.- TOLERANCE - ELASTOMERIC DISC.-

E1).- Diameters greater than 20 inches: +3/32"

E2).- Diameters less than 20": +1/16"

E3).- Thickness: -0", +1/8"

F.- TOLERANCE - GUIDE BAR.-

F1).- Length (unless integral): +1/8"

F2).- Flatness of backing surface for stainless steel: Class A

F3).- Inside of bar to inside of bar: Nominal dimension +1/32"

F4).- Guide bars shall be not more than 1/32" out of parallel.

F5).- Cross sectional dimensions: +1/16"

G.- TOLERANCE - POT

G1).- The inside diameter shall be machined to a tolerance of +0.005" for inside diameters up to 20 inches, and to a tolerance of +0.007" for inside diameters over 20 inches.

G2).- Pot underside shall be machined parallel to the inside to a Class A tolerance.

H.- TOLERANCE - PTFE SUBSTRATES.-

H1).- PTFE substrate flatness: Class A

I.- TOLERANCE OF STEEL (not stainless), in contact with steel (not stainless): Class B

J.- THE EDGES of all parts shall be broken by grinding so that there are no sharp edges.

K.- TOLERANCE - OVERALL HEIGHT OF BEARINGS: -1/16", +1/8"

L.- TOLERANCE - MAKEUP PLATE.-

L1).- Plan dimensions: -0", +1/4"

L2).- Thickness: -1/32", +1/8"

L3).- Flatness: Class B Top and Bottom.

M.- TOLERANCE - MASONRY PLATE.-

M1).- Plan dimensions: -0", +1/4"

M2).- Thickness: -1/32", +1/8"

M3).- Flatness - Class C for the underside, Class B for the upper side.

4.- TESTING:

4.1) GENERAL.-

(4.1.1) Tests shall be performed by the manufacturer or by an independent testing laboratory. The testing agent chosen by the contractor will be subject to approval by the Director. Approval will be based on: (1) The ability of the testing facility to perform the required test - possession of proper testing equipment and trained personnel, and (2) submittal of a report describing the testing procedures to be used including setup of testing apparatus, steps to be followed in the testing apparatus, steps to be followed in the testing procedures, readings, conversions of readings to final data, and sample calculations showing how final results are obtained from raw data.

(4.1.2) SAMPLING.-

A) One guided expansion bearing shall be chosen, selected at random, from each applicable lot of completed bearings.

B) One lot shall consist of no more than 50 expansion bearings of one load category.

C) One load category shall consist of bearings having vertical load capacity within a range of no more than 500 Kips.

4.2) FRICTION TEST shall be performed on expansion bearings samples chosen as described in section (4.1.2) above.

(4.2.1) The tests shall be conducted at the maximum working stress for the bearing with the load applied continuously for 12 hours prior to measuring the friction. Maximum working stress shall be determined by dividing the maximum vertical force (obtained from the plans) by the areas of PTFE used on top of the piston. Lateral design load to be applied concurrently on guided bearings.

(4.2.2) The static and dynamic coefficient of friction shall be determined. A sliding speed of less than one inch per minute shall be used. The coefficient of friction thus determined shall not exceed 0.04.

4.3) PROOF LOAD TEST shall be performed on one sample chosen as described in 4.1.2 above in accordance with AASHTO 18.3.5.3.1 Division II

4.4) ADHESION between the PTFE and substrate shall be tested on a test specimen in accordance with ASTM D429, Method B. The minimum peel strength shall be 25 Lbs. per inch. This test is in addition to adhesion determined under 4.2 and 4.3.

4.5) TEST RESULTS shall be presented in a report showing raw test data, reduced test data, sample calculations, and final results along with photographs and conclusions.

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5	OHIO	

83
182

HAM-471-00.25

4.6) CERTIFIED TEST data for all stainless steel, ASTM A709 structural steel and PTFE shall be furnished to the Director showing compliance with the requirements of this specification.

4.7) THE DIRECTOR may require additional bearings to be tested even though required bearing tests have been acceptable. Such additional tests will be paid for under Item 516, ADDITIONAL BEARING TEST, STEEL POT BEARING.

5.- SHIPPING AND PACKING.-

5.1) Bearings shall be securely banded together as units so that they may be shipped to the job site and stored without relative movement of the bearing parts or disassembly at any time. This requirement does not apply to the masonry plate or 1/8" sheet lead or preformed bearing pad, which shall be shipped for separate installation. Bearings shall be wrapped in moisture proof and dust proof material to protect against shipping and job site conditions.

5.2) Care shall be taken to ensure that bearings at the job site are stored in a dry, sheltered area free from dirt or dust until installation.

5.3) Centerlines shall be marked on appropriate bearing parts for checking alignment in the field and be shown on shop drawings.

5.4) Each bearing, masonry plate and pad shall have a mark number and the mark number and placement location shall be shown on the shop drawings.

6.- INSTALLATION.-

6.1) Field welding of bearing to masonry plate shall meet the requirements of (3.5.1).

6.2) Bearings shall be evenly supported over their upper and lower surfaces under all erection and service conditions.

6.3) Align the centerlines of the bearing assembly with those of the sub-structure and superstructure. Align the bearings to allow for the designated expansion direction of the structure.

6.4) Erection bars shall be fastened to the truss bottom chord or truss end post to accurately position plates. The guide bars must be in parallel with truss or centerline of bearings. Tolerance in setting the guide bars shall be 1/32" in the length of the bar out of parallel.

6.5) Bearing straps or retaining clamps shall be left in place as long as possible to ensure parts of bearings are not inadvertently displaced relative to each other.

6.6) Set offsets of upper and lower bearing parts to compensate for ambient temperature and as required by plans.

6.7) Field paint exposed steel System IZEU.

7.- METHOD OF MEASUREMENT.-

7.1) A complete and acceptable bearing system furnished and installed including bearing, masonry and sole plates, bearing pad and anchor bolts will be measured on an each basis for each design capacity.

7.2) Additional bearing test if required by the Director will be measured on an each basis for a successfully tested and accepted bearing. Test resulting in a rejected bearing will not be measured and paid for.

8.- BASIS OF PAYMENT.-

8.1) Payment will be made at the unit price bid per EACH for Item 516 "STEEL POT BEARING, AS PER PLAN (___ Kips)", for a complete bearing system designed, furnished and installed in accordance with Item 516 and the plans.

8.2) Payment for all work listed under testing and acceptance of this specification will be made at the bid price per EACH for Item Special, "BEARING TEST, STEEL POT BEARING"

8.3) Additional bearing tests required by the Director will be paid for at the price bid per EACH, listed under Item special, "ADDITIONAL BEARING TEST, STEEL POT BEARING".

Pflum, Klausmeier & Gehrum		9/108
Consultants		OHIO

GENERAL NOTES 3/5

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY		Sta. 30+55.40
		Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
	IEH April 96	
REVISED		

GENERAL NOTE 1

GENERAL NOTES Cont...

F. H. V. A. REGION	STATE	PROJECT
5	OHIO	

84
182

HAM-471-00.25

ITEM 202. - PORTIONS OF STRUCTURES REMOVED, OVER 20 FOOT SPAN, AS PER PLAN.
DESCRIPTION: This work shall consist of the removal of concrete deck including sidewalks, parapets, railings, deck joint steel armor and other appurtenances from steel supporting systems (beams, girders, trusses, crossframes, etc.). This work shall also include the removal of tie plates and brackets, bearing devices scheduled to be retrofitted or replaced (roller nests, masonry plates, pins, steel shoes, etc.), and substructure concrete in areas scheduled to be rebuilt, repaired, or patched. Care shall be taken during removals to protect portions of such systems that are to be salvaged and incorporated into the proposed structure. In this respect, the use of explosives, headache balls and/or hoe ram type of equipment is prohibited. Netting now installed to prevent existing spalled concrete from falling, shall be removed and disposed of. The cost of removal and disposal shall be included as part of this item.

PROTECTION OF TRAFFIC: Prior to demolition of any portions of the existing superstructure, the contractor shall submit his plans for the protection of traffic (vehicular, pedestrian, etc.) adjacent to and/or under the structure to the Director for approval. These plans shall include provisions for any devices and structures that may be necessary to ensure such protection. Vertical clearances specified on the plans or in the proposal shall be maintained at all times except as otherwise approved by the Director.

PROTECTION OF STEEL SUPPORT SYSTEMS: Before deck slab cutting is permitted, the outline of primary steel members in contact with the bottom of the deck shall be drawn on the surface of deck. Small diameter pilot holes shall be drilled 2 inches outside these lines to confirm the location of flange edges. Deck cuts over or within 2 inches of flange edges shall not extend lower than the bottom layer of deck slab reinforcing steel. Cuts made outside 2 inches of flange edges may extend the full depth of the deck. During cutting of the deck slab, care shall be taken not to damage steel members that are to be incorporated into the proposed structure.

REMOVAL METHODS: Concrete may be removed by cutting and by means of hand operated pneumatic hammers employing pointed or blunted chisel type tools. For removals above steel members, a hammer heavier than 35 pounds but not to exceed 90 pounds may be used at the approval of the Engineer, to ensure adequate depth control and to prevent nicking or gouging the primary steel members.

LOADING LIMITATIONS: No part of the structure shall be subjected to unit stresses that exceed 136.5% of the allowable unit stress given in the AASHTO Standard Specification for Highways Bridges due either to demolition, erection or construction methods, or to the use of movement of demolition or erection equipment on or across the structure. Structural analysis computations, by a Registered Professional Engineer, showing the allowable stresses and the maximum stresses produced by the Contractor's methods or equipment shall be submitted to the Director for review and approval at least two weeks prior to the start of the work.

DAMAGE REPAIR: Any damage to portions of the structure that are to remain and be incorporated into the proposed structure shall be repaired at the Contractor's expense. Proposed repairs shall be developed by a registered Professional Engineer and submitted in writing for review and approval by the Director.

PAYMENT: This work will be paid for at the contract lump sum price bid for Item 202, PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN, and payment shall be full compensation for all labor, equipment, material, and incidentals necessary to complete the work in conformance with these requirements, with pertinent provisions of 202, and to the satisfaction of the Engineer.

ITEM 516. - JACKING AND TEMPORARY SUPPORT OF SUPERSTRUCTURE, AS PER PLAN. This item shall consist of furnishing all necessary labor, materials, and equipment to raise or reposition any existing structure to the dimension and requirements defined in the project plans.

The Contractor shall be responsible for the design, installation and operation of an adequate jacking system, including any temporary or permanent supports necessary to perform the work described in the project plans. Three (3) sets of jacking plans, which include the information described in this note, shall be submitted to the Director for approval at least thirty (30) days before actual work is to begin. The plans shall be prepared and stamped by a Registered Professional Engineer.

Jacking submittals shall include at least the following:

- 1.- The signature and number, or professional seal, of a Registered Professional Engineer who prepared the submittal.
- 2.- Calculations and analysis of the structure to determine and define the actual loading applied at the Contractor's selection jacking points.

- 3.- A drawing showing the physical and dimensional position of the jacks with respect to the structure including clearances and center of lift.
- 4.- A schematic layout of jacks, check valves, pumps with 3 way retractor valve, pressure gages, flow control valves, etc. in accordance with manufacturer's recommendations. All jacks for each abutment or pier shall be connected together. All jacks at each abutment or pier shall be the same size.
- 5.- Analysis and calculations of the stresses induced or created in the structure and any temporary or permanent supports. Design calculations for any temporary or permanent supports.
- 6.- Physical dimensions, materials, and fabrication details of any temporary or permanent supports. Horizontal and vertical movement restraint shall be provided.
- 7.- A step by step procedure detailing all steps in the jacking operation.
- 8.- Method of attachment of structural members. Welding to tension areas will not be permitted.

The entire system including jacks shall have 20% more capacity than required based on calculated loads.

For lifts greater than 1 in., jacks shall have locking nuts to positively lock and support the structure during the lift.

Jacks shall have a swivel load cap, a domed piston head or some other device to protect against the effects of side load on the jack.

Jacks alone shall not be used to support loads except during the actual jacking operation. Temporary supports, blocking or other methods approved by the Director shall be used.

Single acting rams with no over-travel protection system shall not be used.

Spare equipment shall be available on site for the required structure raising to proceed in the event of breakdown. A list of spare equipment shall be provided to the Engineer.

At a minimum, a jacking operation shall lift all beams or trusses at any one abutment or pier simultaneously. The only exception is the situation where the work involves replacing or rehabilitating individual bearings: no permanent shimming is required and the height of the lift shall not exceed 1/4 in.

Maximum differential jacking height between any adjacent abutments of piers shall be 1 in., or less.

All structure jacking must be completed and the superstructure in its final position prior to placing concrete for the deck

The Contractor shall demonstrate to the Engineer that the bridge bearings are fully seated between all contact areas. If full seating is not attained, suitable means of repair, subject to the approval of the Engineer, will be required at the Contractor's expense.

The jacking operation shall be directed by a Professional Engineer employed by the Contractor. Failure to have a Professional Engineer present shall be cause for ceasing jacking operations.

Payment shall be made at the lump sum price bid for Item 516, Jacking and Temporary Support of Superstructure, As Per Plan and shall include all necessary tools, labor, equipment and materials necessary to complete this item of work.

CUT LINE CONSTRUCTION JOINT PREPARATION:

Saw cut boundaries of proposed concrete removals 1" deep. Remove concrete to a rough surface. Where practicable, the existing reinforcing steel where required in the plans shall be left in place. Install dowel bars if specified. Prior to concrete placement abrasively clean joint surface and exposed reinforcement to remove loose and disintegrated concrete and loose rust. The joint surface and exposed reinforcement shall be thoroughly cleaned of all dirt, dust, or other foreign material by the use of water, air under pressure, or other methods that produce satisfactory results. Concrete bonding surfaces shall be wet without free water as concrete is placed.

EXISTING REINFORCING STEEL. -

Existing reinforcing steel exposed by concrete removals and noted to be incorporated into the new work shall be left in place except that it shall be bent as necessary to clear proposed concrete surfaces by at least 2 inches.

REPLACEMENT OF EXISTING REINFORCING STEEL: Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by concrete removal operations shall be replaced with new steel at the Contractor's cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 800 pounds is included in Item 509 for this purpose, listed in the "General" column of the Estimated Quantities table.

ITEM 513-STRUCTURAL STEEL-REPLACEMENT OF DETERIORATED END CROSS FRAMES, AS PER PLAN.

DESCRIPTION: This item includes all labor, material, equipment, tools, and incidentals necessary to remove deteriorated structural steel end cross frames from the existing main superstructure and replace with new steel end cross frames as described in the plans.

Steel members to be fabricated under this item will not require shop drawings prior to fabrication. The Contractor shall make necessary measurements and prepare sketches, drawings, tables, etc. The Engineer shall have authority and responsibility for ensuring that the fabricated steel is acceptable. Technical assistance will be provided on request by the Bureau of Bridges. Mill test reports and shipping documents shall be submitted to the Engineer for review and approval prior to incorporating steel items into the work, as required by CMS 501.07.

After fabrication, the Contractor shall submit shop drawings to the Engineer for review and approval to ensure that the drawings depict the steel as actually incorporated into the work. The Engineer will then send one approved set to the Bureau of Bridges for information. Pay weights shall be computed in compliance with 513 of the Construction and Material Specifications (CMS), and submitted to the Engineer for his review and approval. The fabricator shall furnish a 35 millimeter microfilm copy of each shop drawing, which shall be mounted on an aperture card as specified in CMS 501.05.

Steel members described in this item include vertical and horizontal gusset plates, steel angles, lacing bars, filler plates, bolts, nuts, washers, etc.

MATERIALS: The new steel shall be structural steel ASTM A709 grade 36 and all fasteners shall be high strength bolts 7/8 in. diameter, ASTM A325. All bolts shall be installed as per CMS 513.15

METHOD OF PAYMENT: The pay quantity will be based on the weight in pounds of each end frame installed, inspected, approved, and accepted by the Engineer.

BASIS OF PAYMENT: Payment will be made at the Contract unit price bid per pound for Item 513-STRUCTURAL STEEL, REPLACEMENT OF DETERIORATED END CROSS FRAMES, AS PER PLAN.

REPLACEMENT OF DETERIORATED STRUCTURAL STEEL:

If the Engineer determines that portions of the existing structural steel are too deteriorated to be reused, the Contractor shall replace such deteriorated steel in kind, with payment at the unit cost bid for Item 513, Structural Steel.

Pflum, Klausmeyer & Gehrm Consultants		10/108
GENERAL NOTES 4/5		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	CHECKED WDD
TRACED	REVIEWED IEH	DATE April 96

GENNOTE4 Scale 1

F. H. W. A. REGION	STATE	PROJECT
5	OHIO	

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182

HAM-471-00.25

ITEM 511-CONCRETE, MISC.: LIGHTWEIGHT CONCRETE SUPERSTRUCTURE, AS PER PLAN.
DESCRIPTION. - This item shall consist of furnishing and placing Portland cement lightweight concrete for the bridge superstructure in accordance with these specifications and in reasonably close conformity with the lines, grades, and dimensions shown on the plans. All applicable provisions of the Specifications shall apply.

MATERIALS. -
 Materials shall be:

FINE AGGREGATE. - Natural sand in accordance with 703.02.

COARSE AGGREGATE. - Lightweight coarse aggregate shall be only processed aggregates prepared from natural deposits such as shales and clays, produced by a rotary kiln operation meeting the requirements of ASTM C330. By-products of industries, such as furnace slag or fly ash, are not acceptable. The coarse aggregate size designation shall be 3/8 inch to No. 8. Damp lightweight aggregates shall be stored and used in a stable, uniform, moist condition. Saturated aggregates, being more vulnerable to freezing and thawing, shall not be used unless the concrete is allowed to lose its excess moisture, after curing, prior to such exposure.

PORTLAND CEMENT. - Portland cement shall be Type I or Type II, ASTM C150.

WATER. - Water shall be free from contaminants such as sewage, oil, acid, strong alkalis or vegetable matter, and also shall be free from clay and loam. Potable water is satisfactory.

AIR ENTRAINING ADMIXTURE. - Air entraining admixture shall be as per CMS 705.10, from approved list on file at the Bureau of Testing.

CHEMICAL ADMIXTURE. - Chemical admixture shall be as per CMS 705.12 from approved list on file at the Bureau of Testing.

PROPORTIONING. -

The Contractor (with the advice of the lightweight aggregate supplier) shall be responsible for designing the lightweight concrete mixture and determining the proportions of cement, fine aggregate, coarse aggregate, water, air-entrainment and chemical admixture, in accordance with the "Recommended Practices for Selecting Proportions for Structural Lightweight Concrete (ACI 211.2)", American Concrete Institute (ACI). The resulting concrete mixture shall meet the following requirements:

28-DAY AIR DRY UNIT WEIGHT:

Design unit weight of 112 pounds per cubic foot with a permissible range of 110 pounds per cubic foot to 115 pounds per cubic foot.

28-DAY AVERAGE COMPRESSIVE STRENGTH:

4,500 psi.

CEMENT CONTENT:

715 pounds per cubic yard (minimum).

SLUMP:

3 inches plus or minus 1 inch.

AIR CONTENT:

6 percent plus or minus 2 percent (entrapped plus entrained).

WATER-CEMENT RATIO:

0.44 maximum, using damp lightweight aggregates.

All lightweight concrete shall have good workability and other properties such that proper placement, consolidation, and finishing are obtained.

The Contractor shall provide certified test data in accordance with CMS 101.061 from a recognized testing laboratory that shows the proposed lightweight concrete mix meets the requirements specified above. The mix shall be tested using the damp loose volumetric method as outlined in ACI 213R-79, Section 3.4.2. A recognized testing laboratory is any laboratory regularly inspected by the Cement and Concrete Reference Laboratory. The proposed mix designs shall be approved by the Director. Any adjustment shall be approved by the Engineer.

FIELD WORK. -

The handling of the lightweight aggregates shall be arranged to provide a thorough sprinkling of the aggregate during the stockpiling to produce damp aggregate. Sprinkling shall be done in such a way as to obtain uniform distribution of moisture. Aggregates shall then be allowed to drain as long as necessary to produce a uniform moisture content, in excess of an SSD condition, and such moisture content shall be maintained insofar as practical until the aggregate is used. The CMS 705.12 chemical admixture shall be added to the mix in strict accordance with the manufacturer's recommendation. The same source of aggregate and design mix, once established and checked, shall be used throughout the whole job.

Variations that exist in batching arrangements, ready mix plant layouts, weather and aggregate properties may require daily minor field adjustments to the batch weights of the aggregate. The lightweight aggregate supplier will provide, at no cost to the producer or owner, a qualified technical representative to the producer of the lightweight concrete during the production period. The technical representative furnished by the lightweight aggregate supplier shall remain available at the production site until such time as the performance of the concrete is proven to the satisfaction of the Engineer.

The dosage rate of chemical (water reducing) admixture to be used in the field shall be per manufacturer's recommendation.

Lightweight concrete shall be placed, tested, finished, water cured, and protected as specified in Item 511. Care shall be taken to provide uniform consolidation without over-vibration. The lightweight concrete shall be water-cured in accordance with CMS 511.14 Method (a).

METHOD OF MEASUREMENT:

The yardage shall be the number of cubic yards determined by calculations from plan dimensions, in place, complete and accepted. No deductions will be made for the volume of the reinforcing steel.

BASIS OF PAYMENT:

Payment shall be made at the contract unit price bid per cubic yard for Item 511, CONCRETE, MISC.: LIGHTWEIGHT CONCRETE SUPERSTRUCTURE, AS PER PLAN.

PAINTING OF STRUCTURAL STEEL: New steel shall be cleaned and prime painted in the shop and field painted with an intermediate and finish coat of paint using System IZEU. Existing steel shall be field cleaned and painted with a prime, intermediate, and finish coat of paint using System OZEU. For pay purposes, cleaning and prime painting new steel is included in 513, intermediate and finish painting of new steel in 514, and field cleaning and painting existing steel in the several OZEU items. The surface area pay quantities are based on the surface of main members, including crossframes increased by 5 percent to account for the area of bearings, and other incidentals being cleaned and painted. Paint color shall be subject to approval by the City Of Cincinnati.

SEALING WITH HIGH MOLECULAR WEIGHT METHACRYLATE (HMWM) RESIN.

After deck slab concrete has been dry air cured for not less than 7 days, and immediately after a minimum 48 hour period without precipitation, vertical construction joints in the deck slab, horizontal joints at and adjacent to the roadway surface (at the base of sidewalks, curbs, barriers, etc.), and cracks in the roadway surface that are visible to the unaided eye, shall be sealed with a HMWM resin as described in the HMWM Proposal Note. Sealant shall be applied by brush, spray, or other suitable applicator along the surface of joints and cracks. If necessary, multiple applications shall be made until complete penetration has been achieved.

After sealant has been cured, it shall be sanded as specified to roughen the sealant surface and restore its suitability for vehicular traffic. For overcoating with a concrete surface sealant or as preparation for a concrete overlay, treated surfaces shall be roughened by abrasive blasting and otherwise cleaned as specified for the subsequent application. Sealing construction joints and cracks, as described above, shall be included with the deck slab concrete for payment.

Construction joints and cracks in overlays and butt joints between overlays and abutting concrete or metal deck elements (i.e., scuppers, end dams, etc.) shall also be sealed. Sealant shall be applied to the fresh overlay, but only after a dry air cure of 7 days, and immediately after a minimum 48 hour period without precipitation. Treated areas in the traveled way shall be sanded as required. Sealing joints and cracks in and adjacent to overlays as described above shall be included with Item 511, Conc. Misc.: Lightweight Concrete Superstructure, As Per Plan, for payment.

BRIDGE IDENTIFICATION SIGN: The bridge identification sign shall be carefully removed prior to demolition work and shall be salvaged for reuse. After completion of proposed bridge work, the sign shall be reinstalled outside of the railing at the right rear corner of the bridge with the bridge number facing traffic. It shall be fastened with cast-in-place or grouted-in-place stainless steel or galvanized steel anchor bolts. The price bid for Item Special, Structure Misc.: Bridge Identification Sign, shall be full compensation for all materials, labor, and incidentals necessary to complete this item.

ITEM SPECIAL-VANDAL PROTECTION FENCE. -

DESCRIPTION: This item consist of furnishing all materials, labor, tools, equipment, and incidentals necessary to fabricate and install a steel fence in accordance with the details shown on sheets 78/108, 79/108, and 85/108.

Fence post shall be set plumb in precast sockets and grouted. The grouting shall be finished with 1/4" wash. Support bar inserts on the side of the designated pylons shall be set level. Both, posts and support bars, shall be anchored with non-shrinking epoxy grout 510.

Fence panels shall consist of a stockade of picket and spike steel bars as shown on Sh. 85/108, arranged and spaced as described thereon.

The fabrication of the stockade shall be such, that once installed, pickets and spikes shall be vertical, and the steel channel chord members shall lay parallel to the top of the concrete railing.

Extreme care shall be exercised in the positioning and anchoring of posts and support bars to achieve this objective. Holes for anchoring posts and support bar inserts shall be satisfactorily protected to prevent water and/or dirt accumulation prior to the erection and installation of the posts and inserts. The Contractor shall submit shop drawings of the fence to the Engineer for approval prior to fabrication. Painting of the fence shall conform to Item 514, Painting of New Steel, System IZEU, retouched after erection, (see Proposal Note). Color of paint shall be black, as approved by the City Of Cincinnati.

METHOD OF MEASUREMENT: The pay quantity will be based on the linear feet of fence, installed, inspected, approved, and accepted by the Engineer.

BASIS OF PAYMENT: Payment for all the above described work will be made at the contract bid price per linear foot for Item Special, VANDAL PROTECTION FENCE (___ft. high).

ITEM 517-RAILING, CONCRETE (DECORATIVE TYPE).

This item consists of furnishing all materials, labor, tools, equipment and incidentals, necessary to construct and erect the complete railing described on sheets 76/108 thru 84/108.

MATERIALS:

Reinforcing steel ASTM A615, A616, A617, Grade 60, epoxy coated. Concrete shall be as per 511, Lightweight Concrete, Superstructure, As Per Plan, modified as follows: Concrete shall be buff colored by means of a water-reducing, set-controlling admixture for architectural concrete in conformance to AASHTO Specification M-194, and ASTM C-494. The dosage rate of the color-conditioned concrete admixture used in designing the lightweight concrete mix shall be the dosage rate specified by the admixture supplier.

SURFACE FINISH. - all surfaces of the railing, including posts and pylons shall be provided an exposed aggregate finish. The Contractor shall submit his proposed method for providing the finish to the Engineer for approval. The Contractor shall submit 12"x12"x2" samples of the colored concrete with exposed aggregate finish for color and texture approval by the Engineer prior to the construction of the railing.

METHOD OF MEASUREMENT: The pay quantity will be based on the length of railing measured along its centerline, inspected, approved, and accepted by the Engineer.

BASIS OF PAYMENT: Payment will be made at the contract unit price bid per linear foot, for Item 517, RAILING, CONCRETE (DECORATIVE TYPE).

ITEM 518.- STRUCTURE DRAINAGE, MISC.: STRAIGHTENING, ADJUSTING, AND CLEANING EXISTING 6" DOWNSPOUT INCLUDING SPECIALS.

DESCRIPTION. - This Item includes all labor, material, equipment, tools, and incidentals necessary to clean and straighten existing downspouts scheduled to be protected during concrete removals, and incorporated into the new work. The work includes removal and replacement of pipe clamps, welds, cleanouts, hangers, etc., damaged during work operations, or if deemed unusable as directed by the Engineer.

The work shall also include the cleaning of all existing cleanout boxes, and connecting pipes. Any cleanout box damaged during removal and insertion of new pipe downspout shall be repaired. Any cleanout box requiring repair as directed by the Engineer shall also be repaired.

All work described above shall be to the satisfaction of the Engineer.

PAYMENT for all the above described labor and material will be made at the Contract price bid LUMP SUM for Item 518, STRUCTURE DRAINAGE, MISC.: STRAIGHTENING, ADJUSTING, AND CLEANING EXISTING DOWNSPOUT INCLUDING SPECIALS.

Pflum, Klausmeier & Gehrum		11/108
Consultants		OHIO

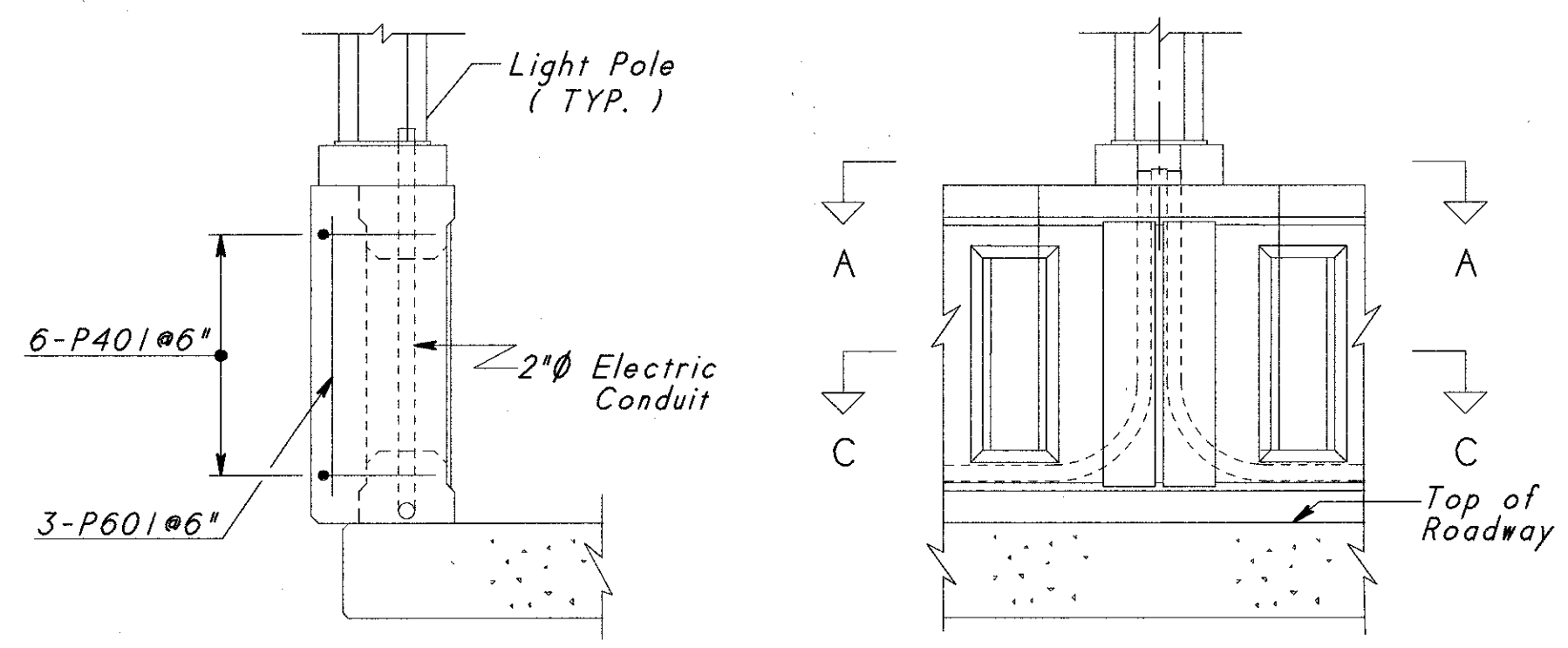
GENERAL NOTES 5/5

BRIDGE No. HAM-471-0025
 (COLUMBIA PARKWAY VIADUCT OVER I-471)

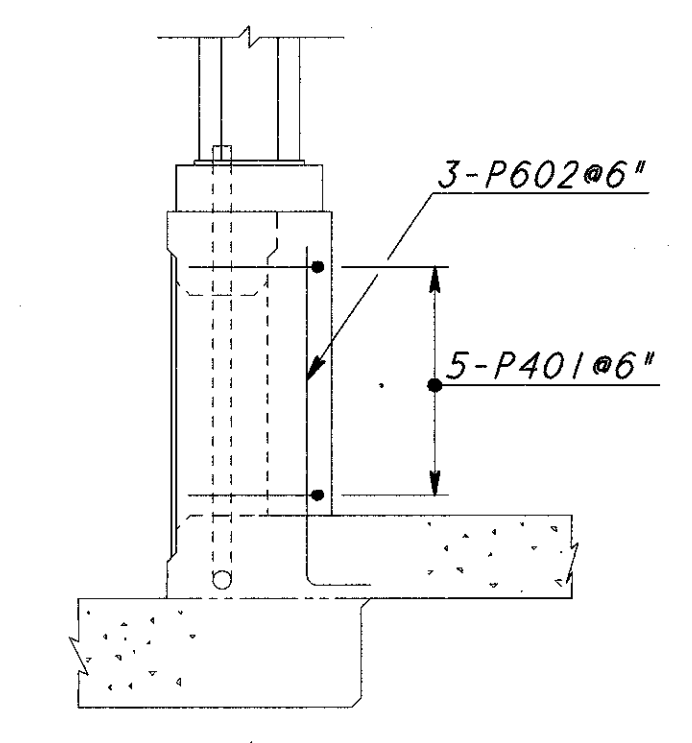
HAMILTON COUNTY		Sta. 30+55.40
DESIGNED	DRAWN	Sta. 47+16.19
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TRACED	CHECKED	REVIEWED
WDD	IEH	April 96

Genote5 Scale 1

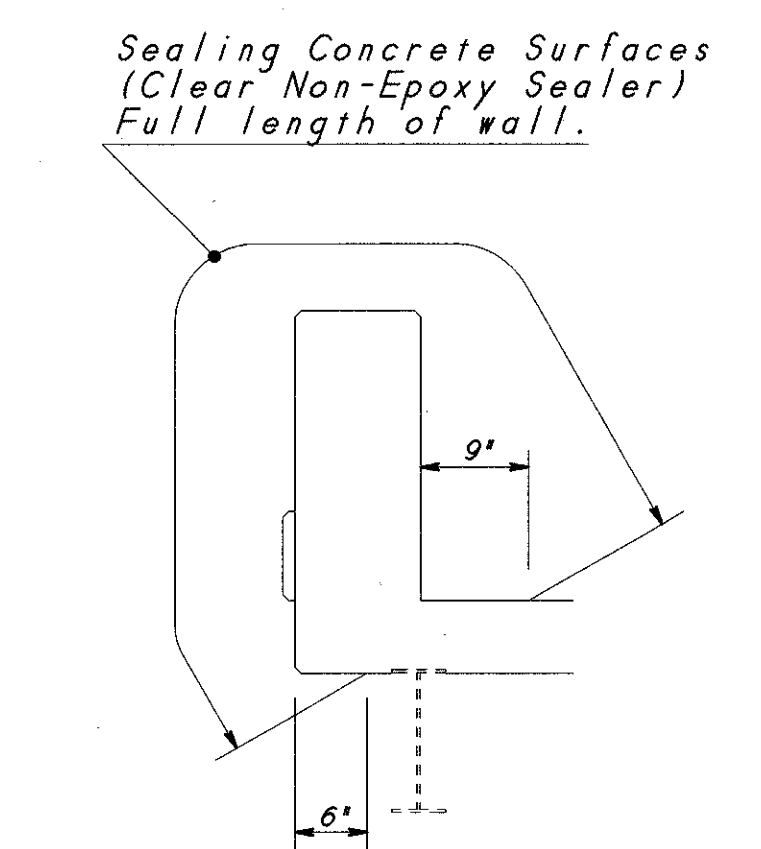
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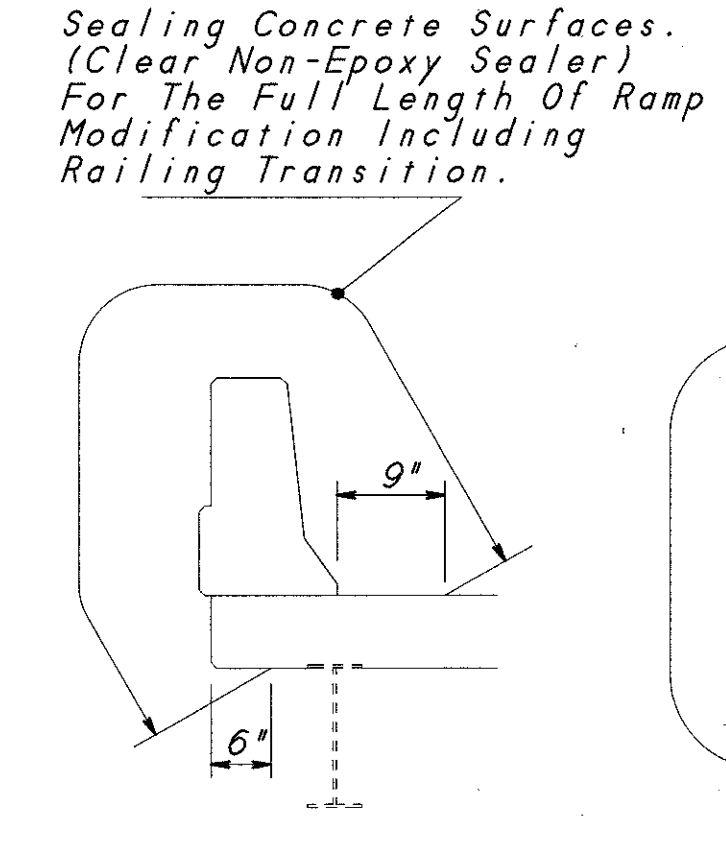
SECTION B-B
(FOR NORTH FASCIA RAILING ONLY)



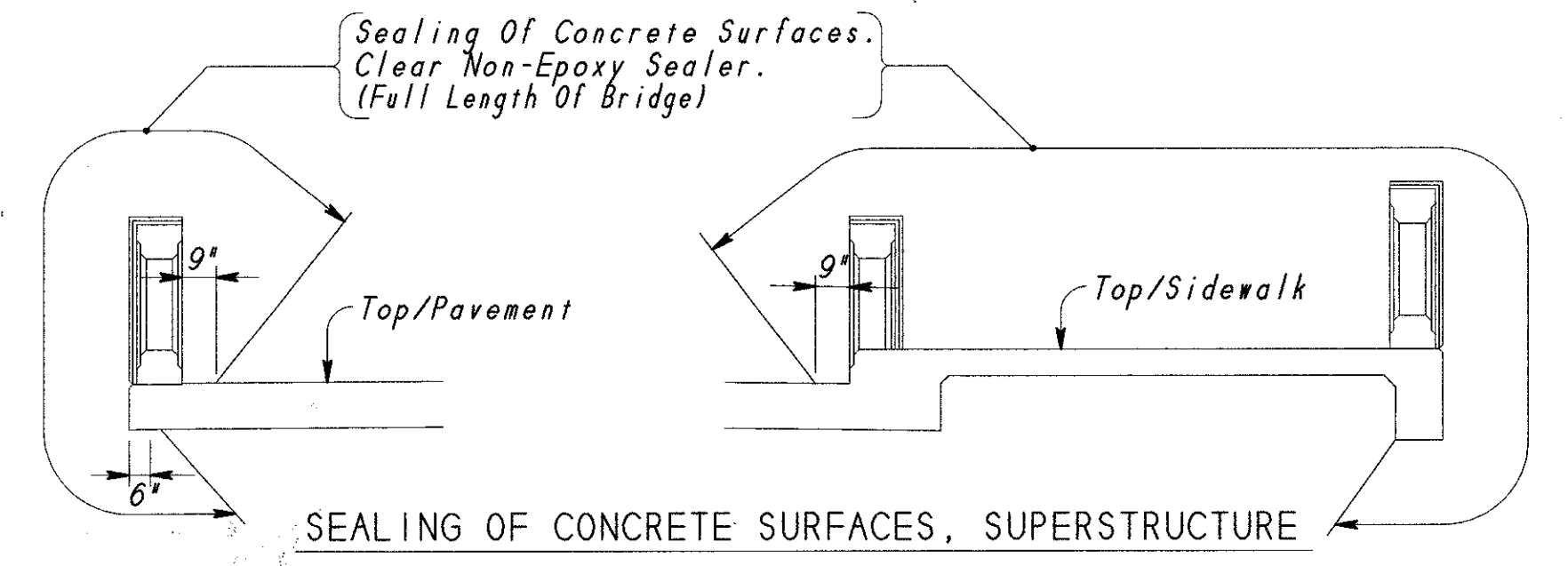
SECTION B1-B1
(FOR SOUTH SIDEWALK CURB RAILING ONLY)



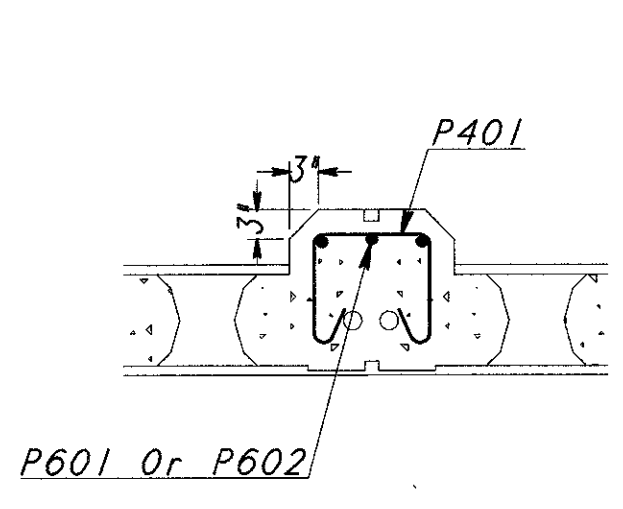
CRASH ATTENUATOR BACK-UP WALL



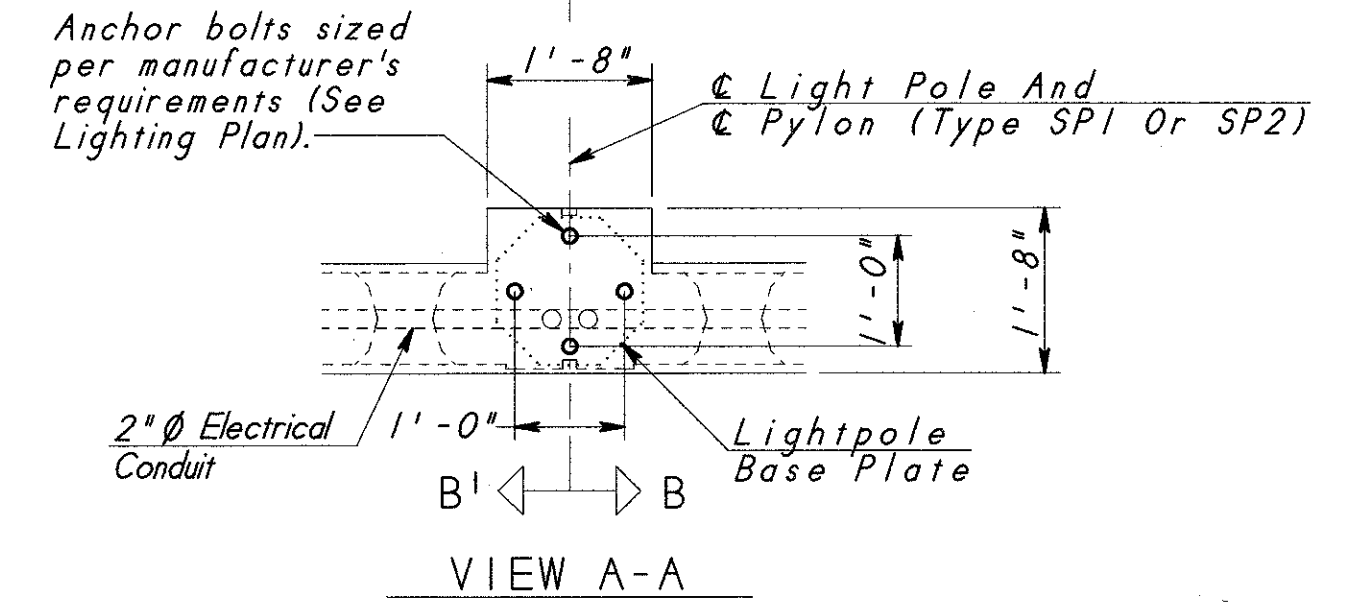
RAILING PARAPET TYPE



SEALING OF CONCRETE SURFACES, SUPERSTRUCTURE

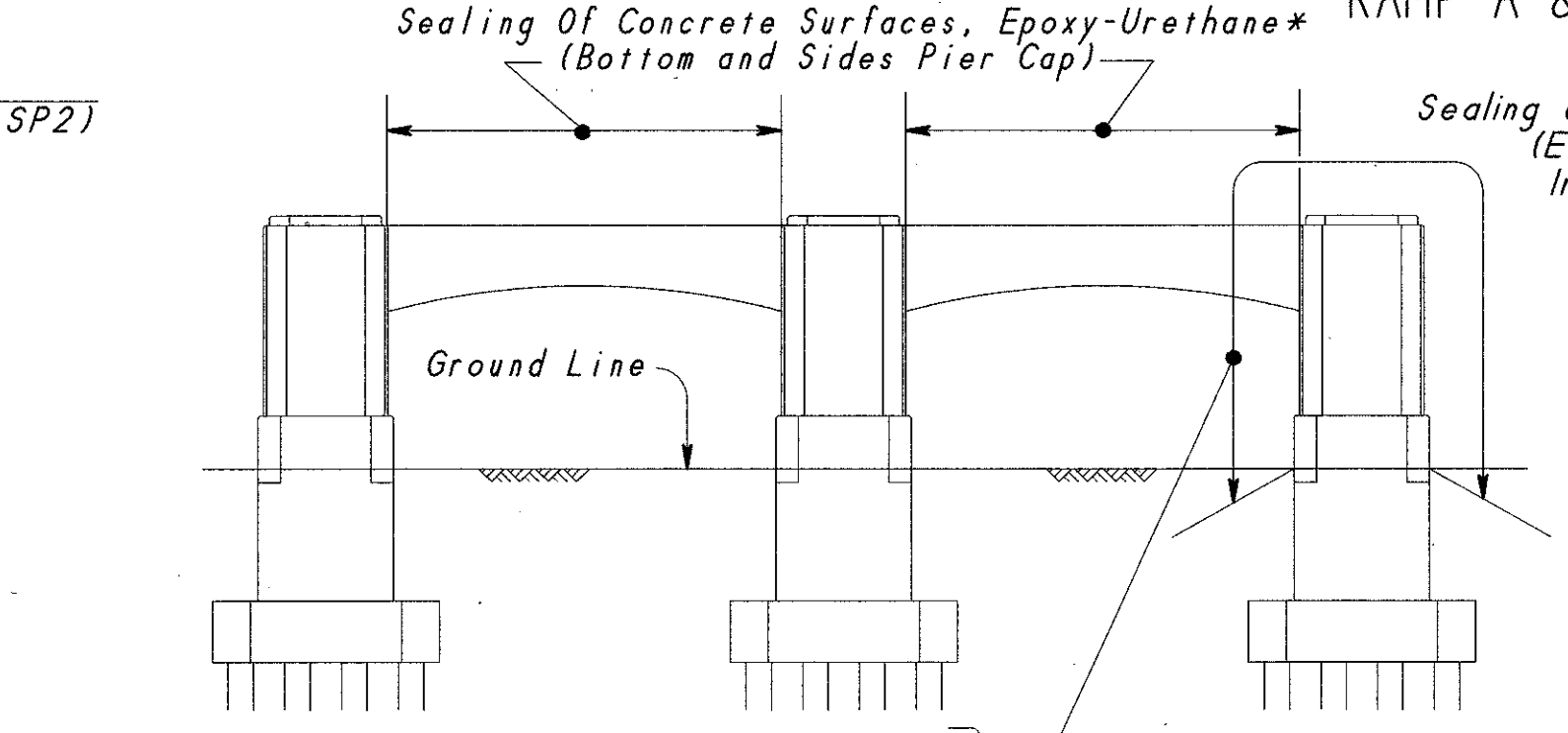


SECTION C-C



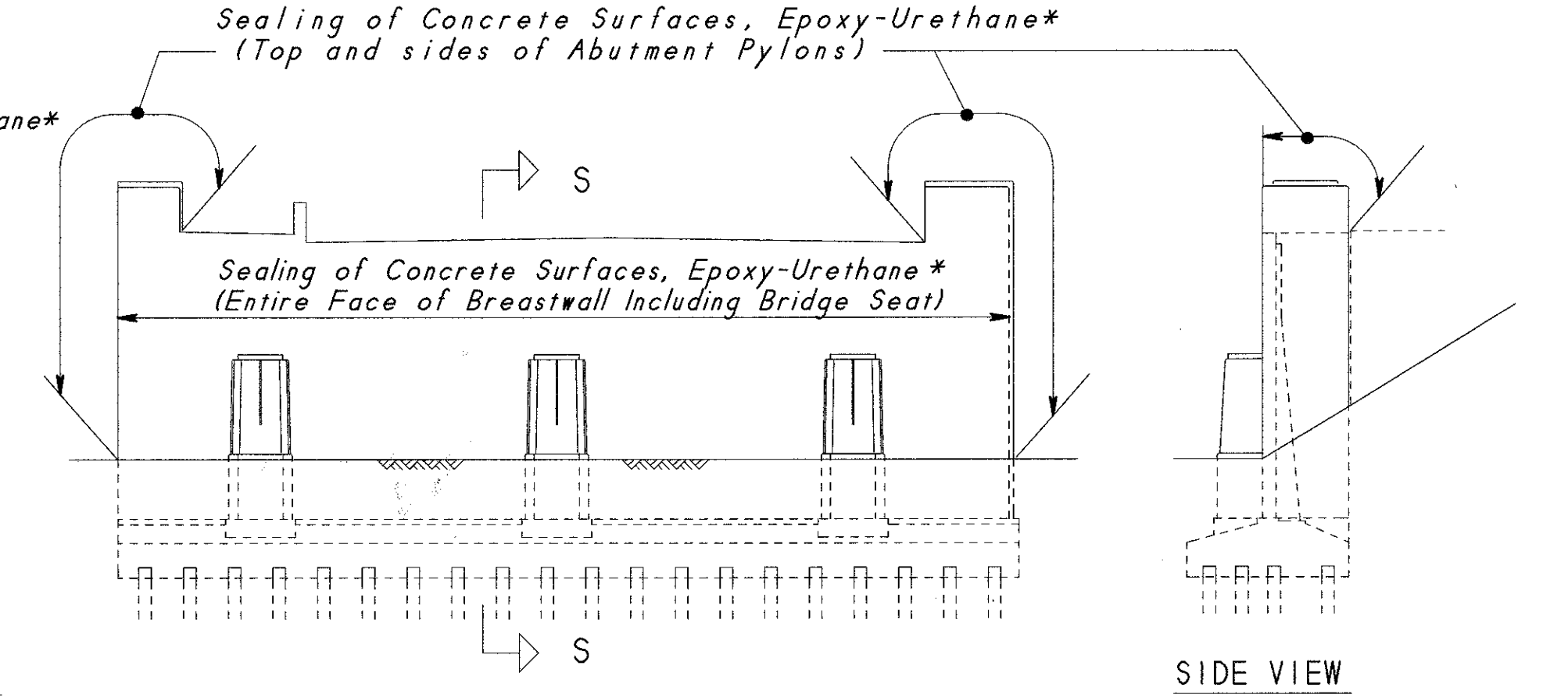
RAILING PYLON MODIFICATION
FOR LIGHTPOLE PEDESTAL

* For Locations See Railing, Sh. 77/108 Thru Sh. 80/108.

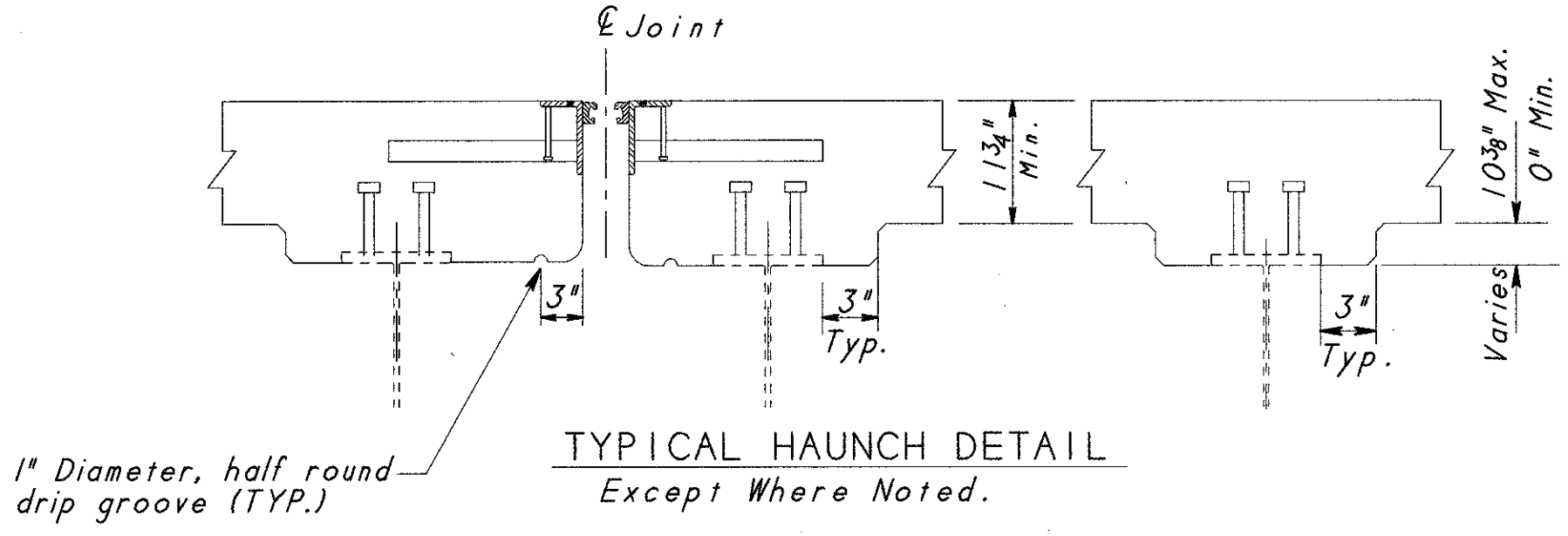


SEALING OF CONCRETE SURFACES, PIERS

SEALING CONCRETE SURFACES
RAMP A & SIXTH ST. CONNECTION



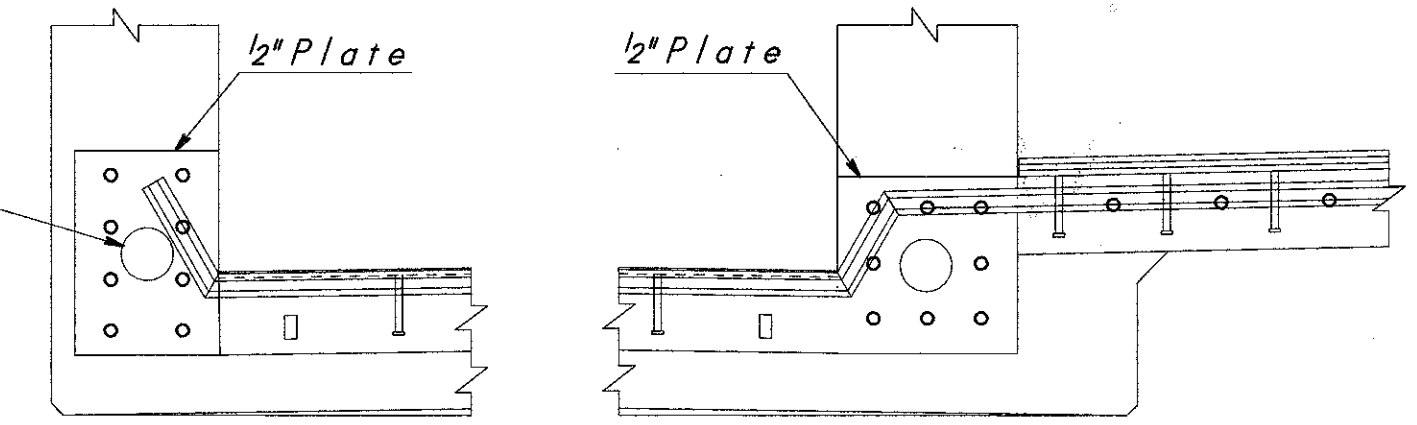
SEALING OF CONCRETE SURFACES, ABUTMENTS
(REAR ABUTMENT ELEVATION SHOWN, FORWARD ABUTMENT IS SIMILAR)



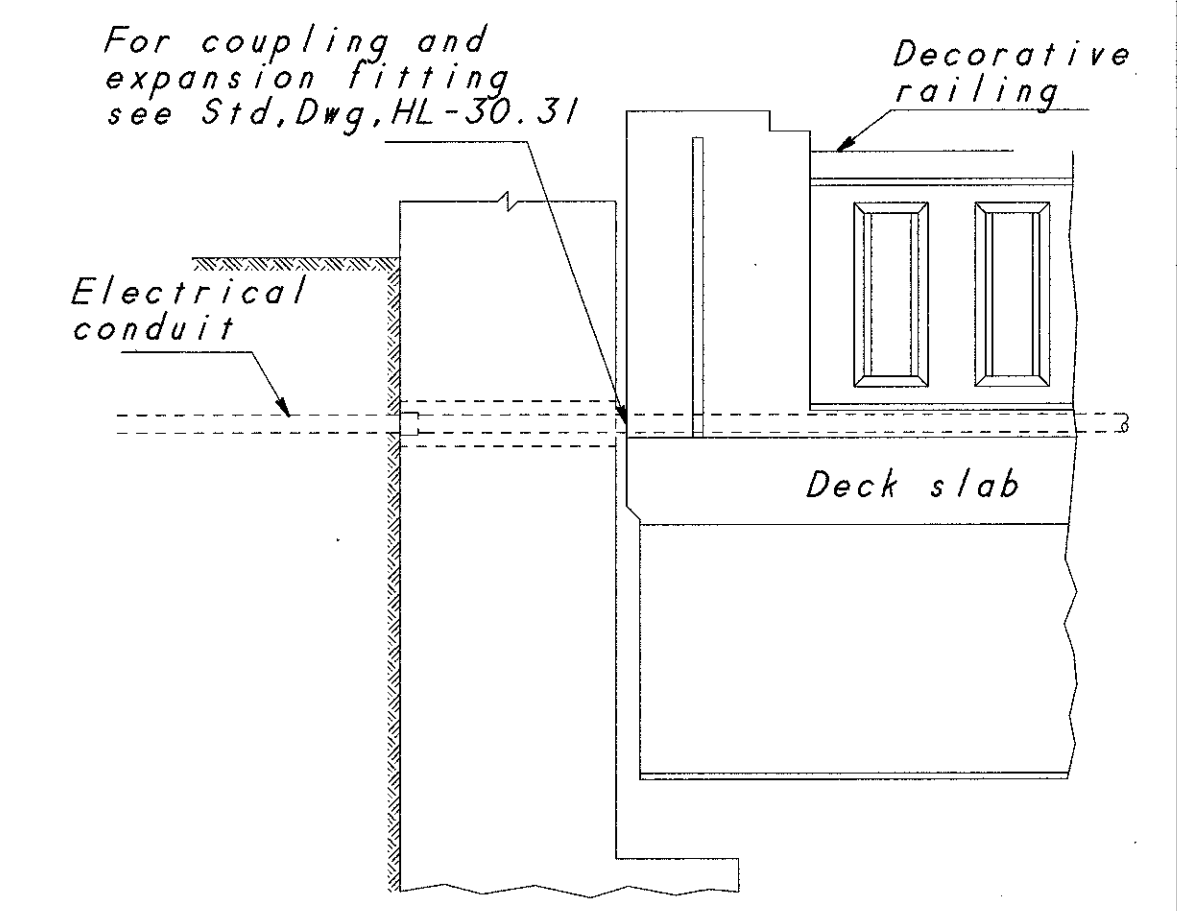
TYPICAL HAUNCH DETAIL
Except Where Noted.

* Sealer shall be a two component Epoxy-Urethane Sealer
The Contractor shall submit a color chart for the epoxy sealer to the Engineer for color selection and approval.

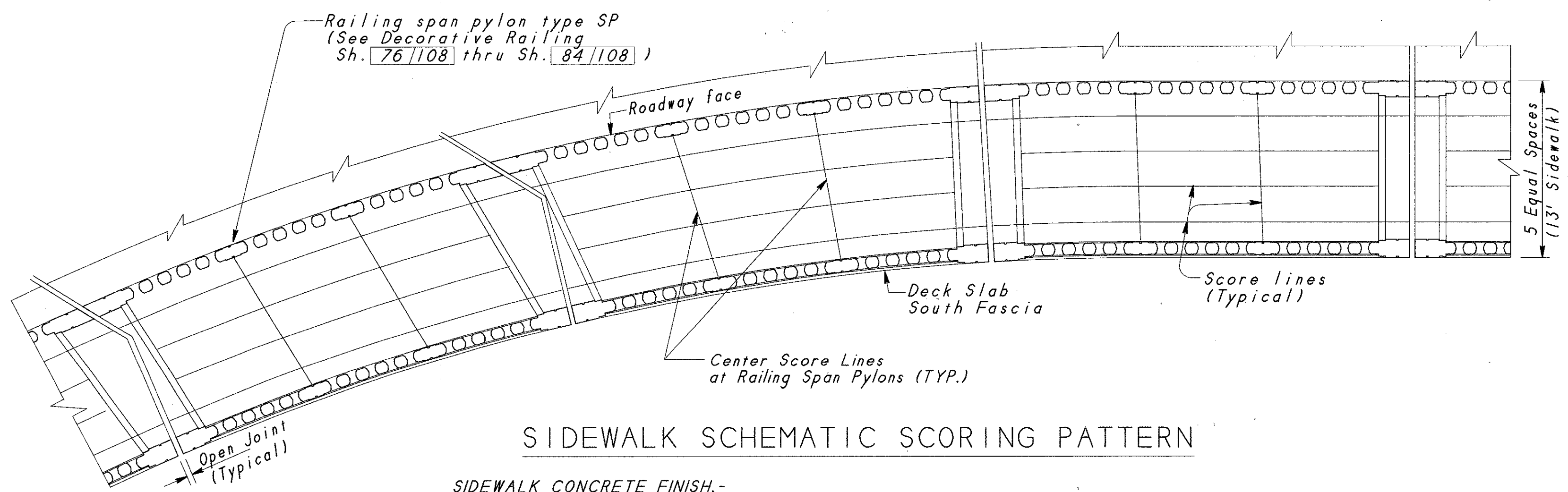
5" hole for conduit and expansion fitting.
See Sh. 53/108 (Typical).



NORTH FASCIA RAILING LINE SIDEWALK CURB RAILING LINE
CONDITION AT EXPANSION JOINT
See STRIP SEAL EXPANSION JOINT
Sh. 53/108 Thru Sh. 59/108



CONDITIONS AT ABUTMENTS



SIDEWALK SCHEMATIC SCORING PATTERN


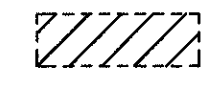
SIDEWALK CONCRETE FINISH.
The sidewalk concrete shall be buff colored. Reference is made to Item 517, Railing, Concrete, (Decorative Type), Sh. 71/108, concerning material. The color will be selected and approved by the Engineer after the contractor submits 12"x12"x2" samples.
The entire sidewalk wearing surface shall receive an exposed aggregate finish and be provided with 1/2" deep score lines as shown in the schematic scoring pattern plan. The Contractor shall submit his proposed method for providing the finish to the Engineer for approval.
All labor, materials, tools, equipment, and incidentals, necessary to complete the work on sidewalk shall be included with Item 511, Concrete, Misc.: Lightweight Concrete Superstructure, for payment.

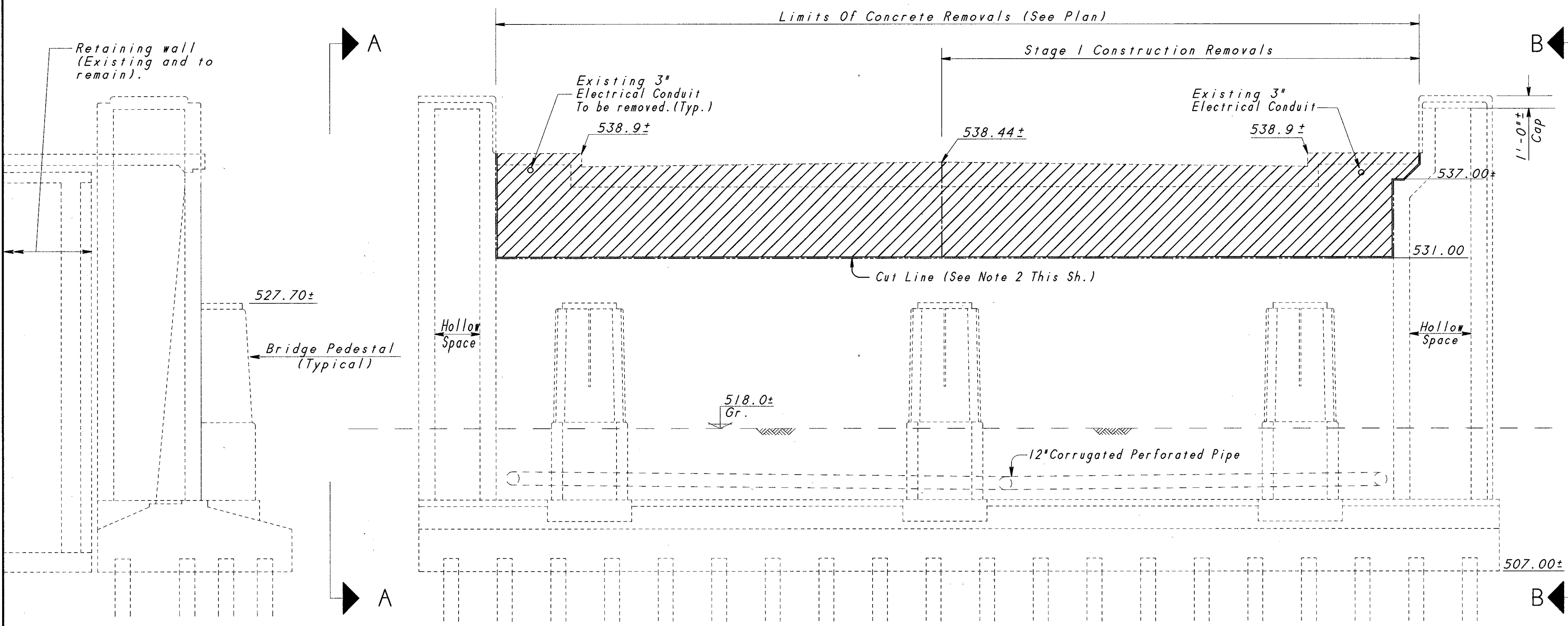
ELECTRICAL CONDUIT LOCATION

Pflum, Klummeier & Gehrum Consultants		12/108
MISCELLANEOUS DETAILS		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN PLF	CHECKED WDD
TRACED	REVIEWED IEH April 96	DATE

MISC Scale 1

LEGEND

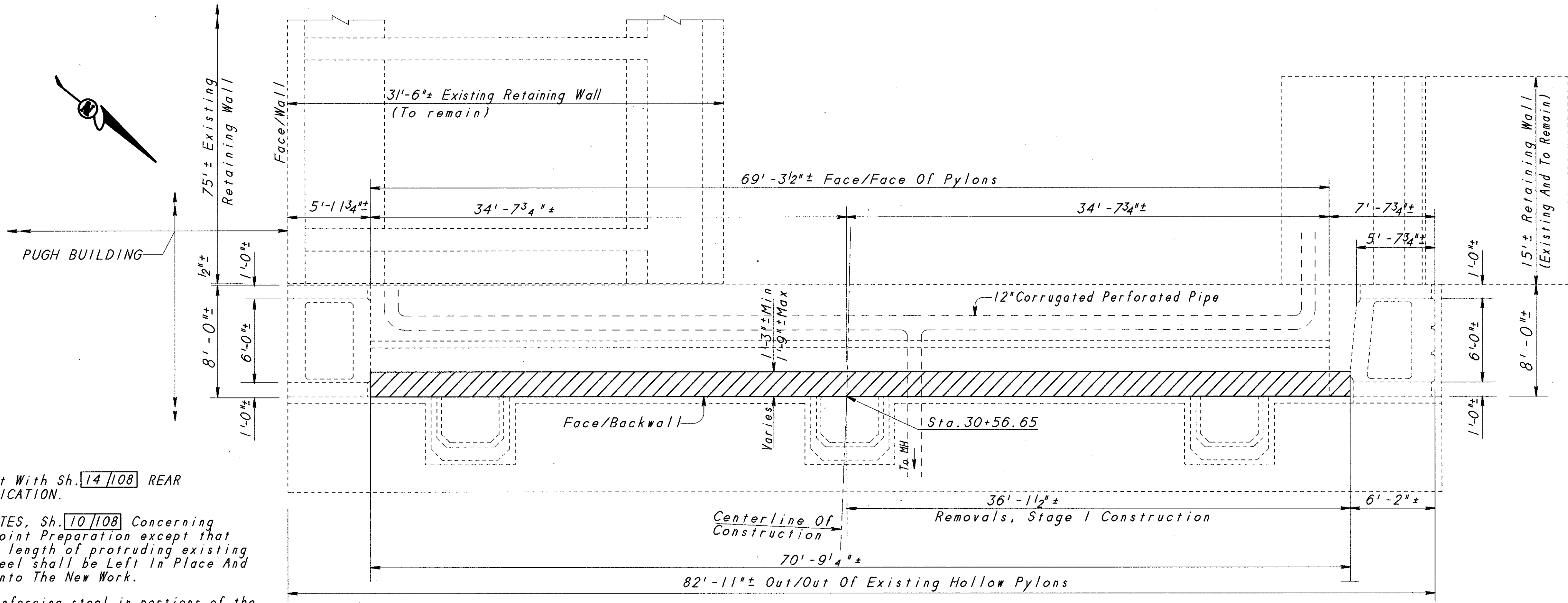
-  Existing Structure To Remain.
-  Concrete Removal



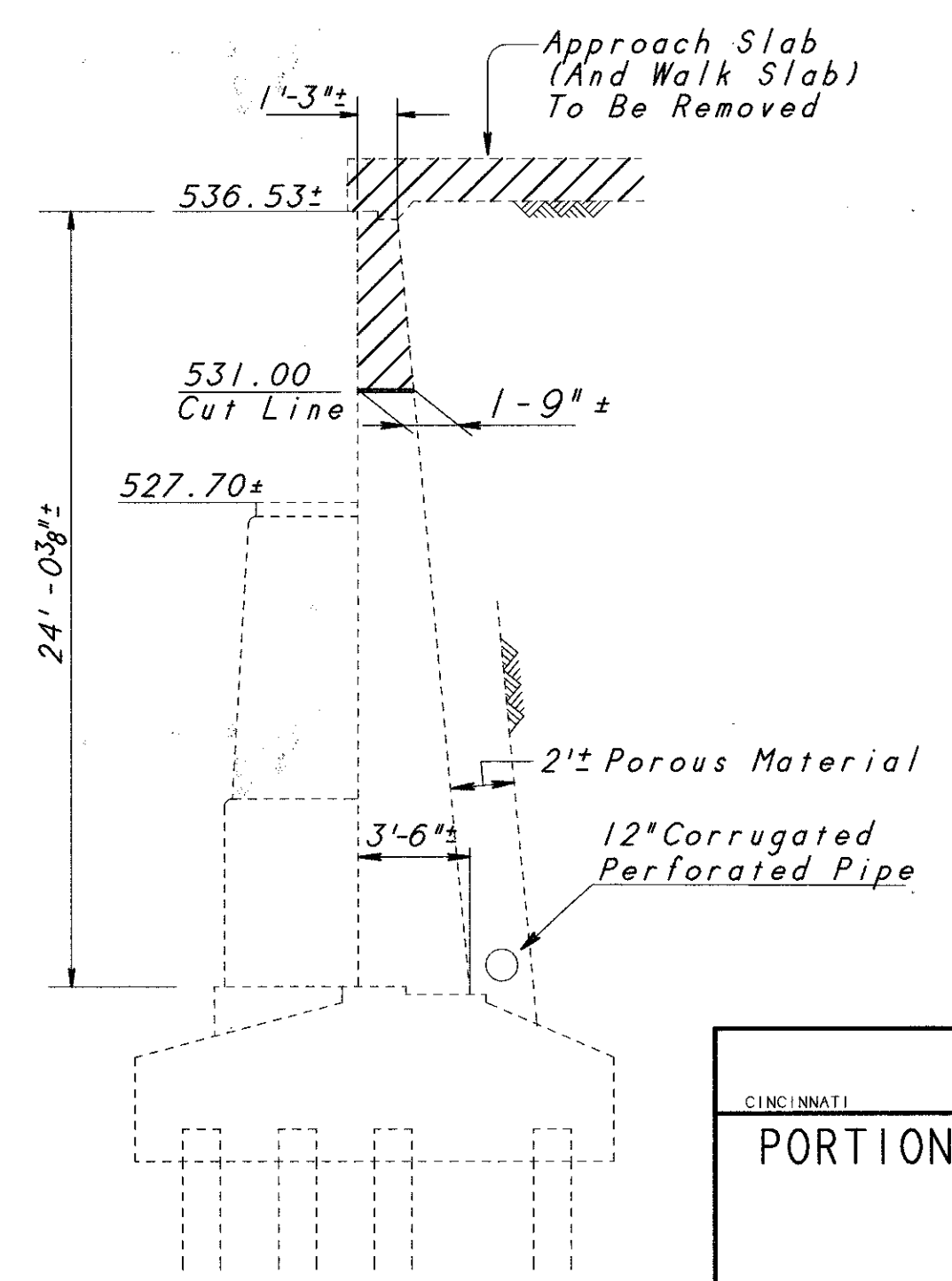
SIDE ELEVATION A-A

ELEVATION

SIDE ELEVATION B-B



PLAN

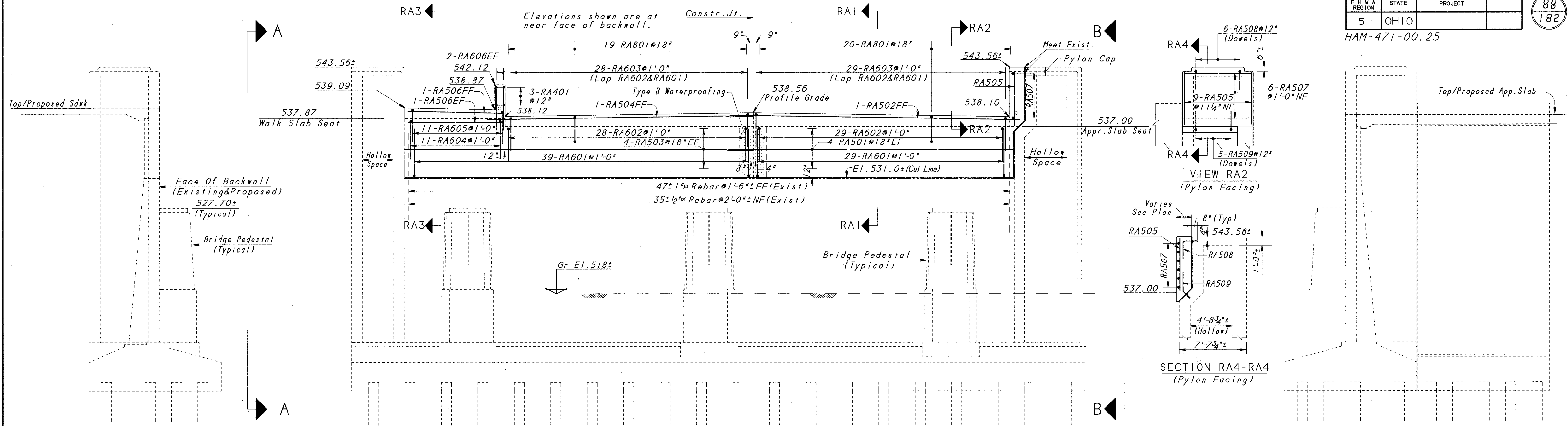


TYPICAL SECTION

- NOTES:
1. - Work This Sheet With Sh. 14/108 REAR ABUTMENT MODIFICATION.
 2. - See GENERAL NOTES, Sh. 10/108 Concerning Construction Joint Preparation except that at least 3'-0" length of protruding existing reinforcing steel shall be Left In Place And Incorporated Into The New Work.
 3. - All existing reinforcing steel in portions of the Abutment Backwall to be removed and replaced shall remain and be protected and be incorporated into the new work.
 4. - REPLACEMENT OF EXISTING REINFORCING STEEL. - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 100 pounds is included in Item 509 for this purpose.

Pflum, Klausmeyer & Gehrum Consultants		13/108
PORTIONS OF STRUCTURES REMOVED REAR ABUTMENT (ABUTMENT No. 1)		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	ED/PLF	WDD
TRACED	REVIEWED	DATE
	EH	April 96

HAM-471-00.25



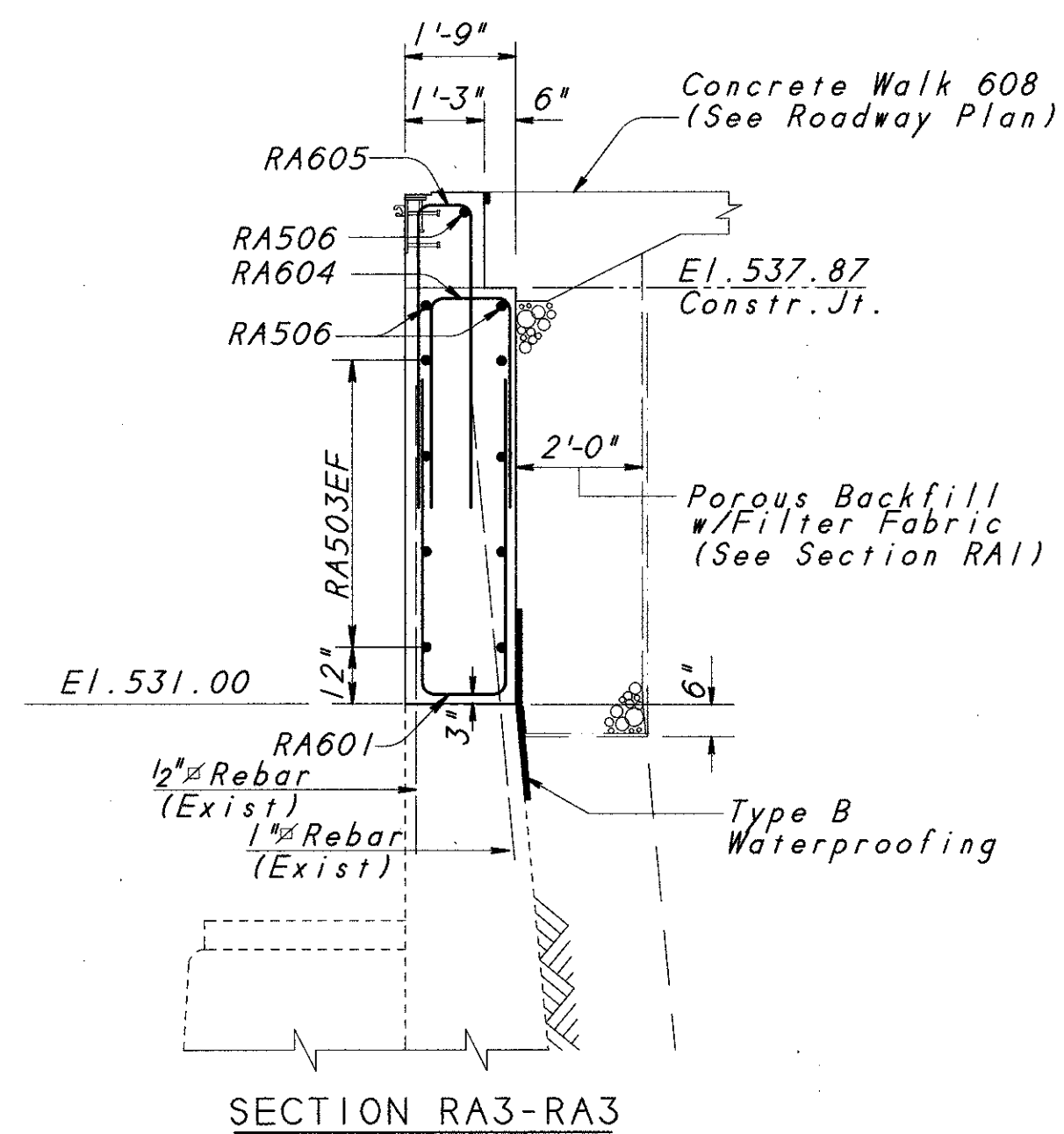
SIDE ELEVATION A-A

ELEVATION

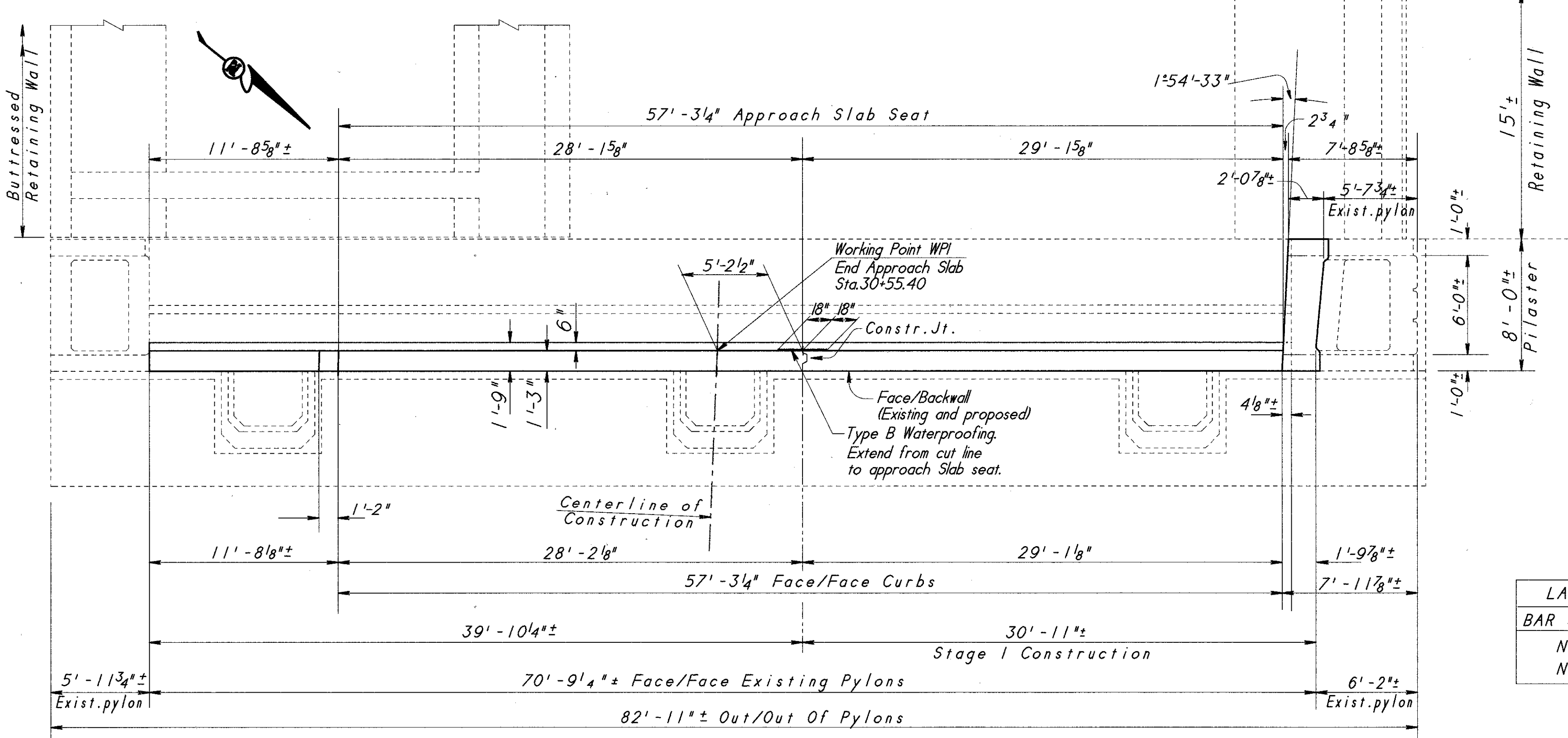
SIDE ELEVATION B-B

LEGEND
 Existing Structure
 New Work
 NF Denotes Near Face.
 FF " Far Face.
 EF " Each Face.
 RA " Rear Abutment rebar.

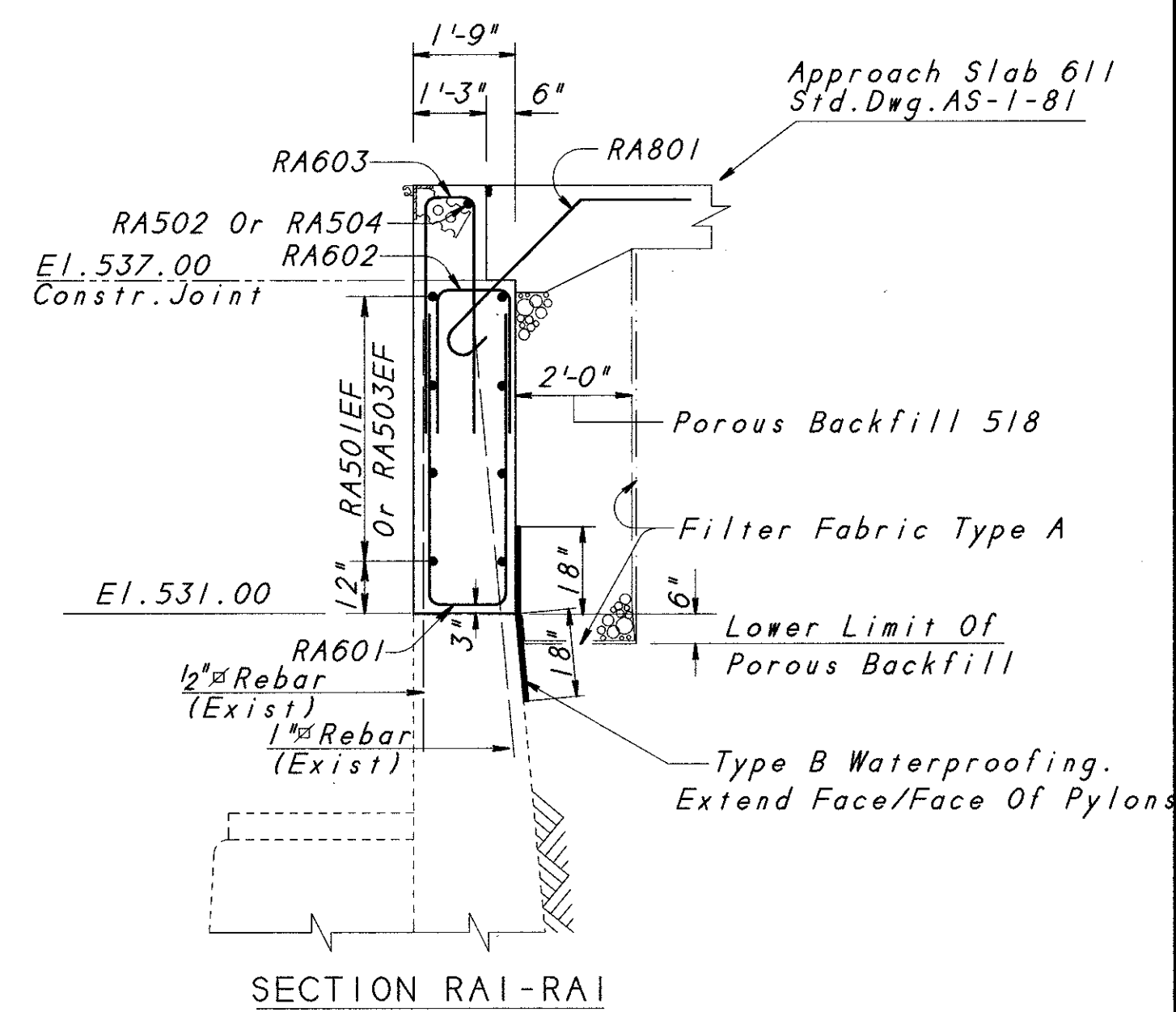
REPLACEMENT OF EXISTING REINFORCING STEEL. - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 100 pounds is included in Item 509 for this purpose and listed in the "General" column of the Estimated Quantities table.



SECTION RA3-RA3



PLAN



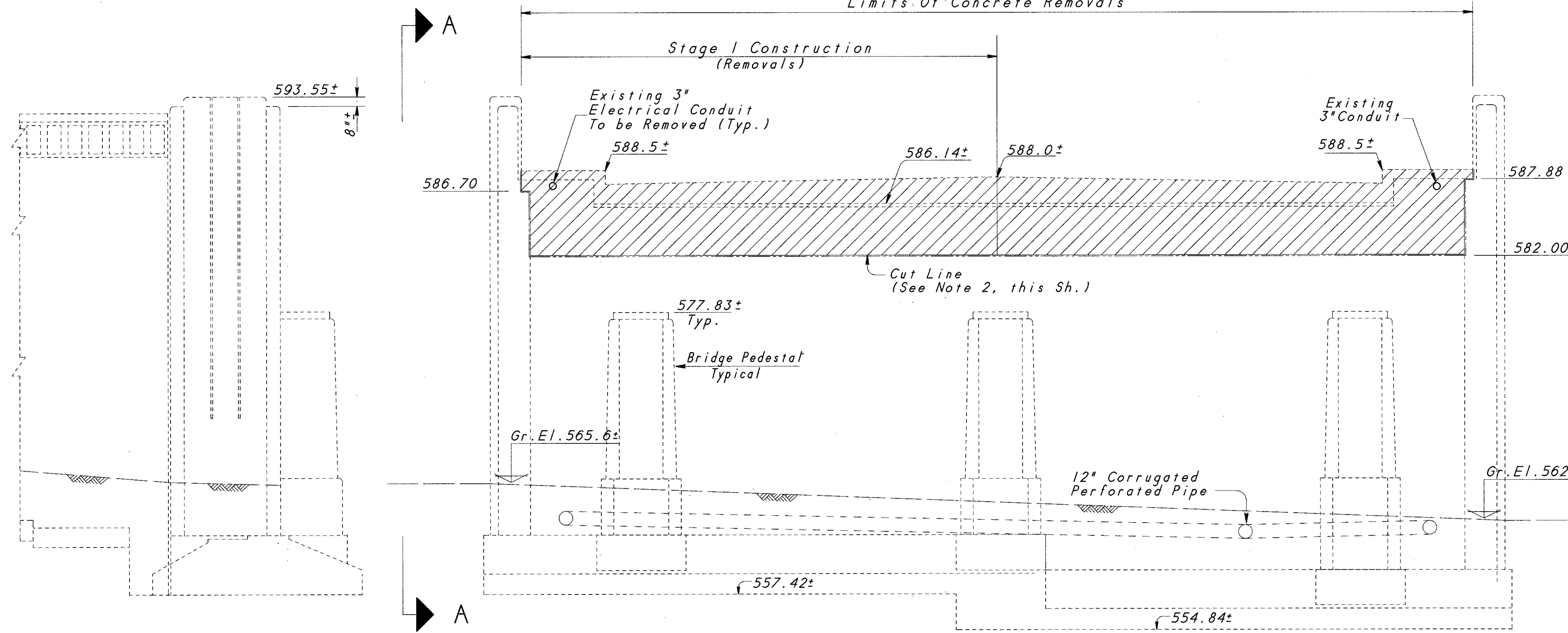
SECTION RA1-RA1

- NOTES:**
- See GENERAL NOTES Sh. 10/108. On Construction Joint Preparation At Least 3'-0" Length Of Protruding Existing Reinforcing Steel Shall Be Left In Place And Incorporated Into The New Work.
 - Work This Sheet With Sh. 13/108 PORTIONS OF STRUCTURES REMOVED, REAR ABUTMENT.
 - See REINFORCING STEEL BAR LIST, Sh. 86/108
 - Clearance Between Reinforcing Steel And Surface Of Concrete Shall Be As Per CMS509 Unless Noted Otherwise.

LAP LENGTH TABLE	
BAR SIZE	LAP LENGTH
No. 5	2'-9"
No. 6	3'-4"

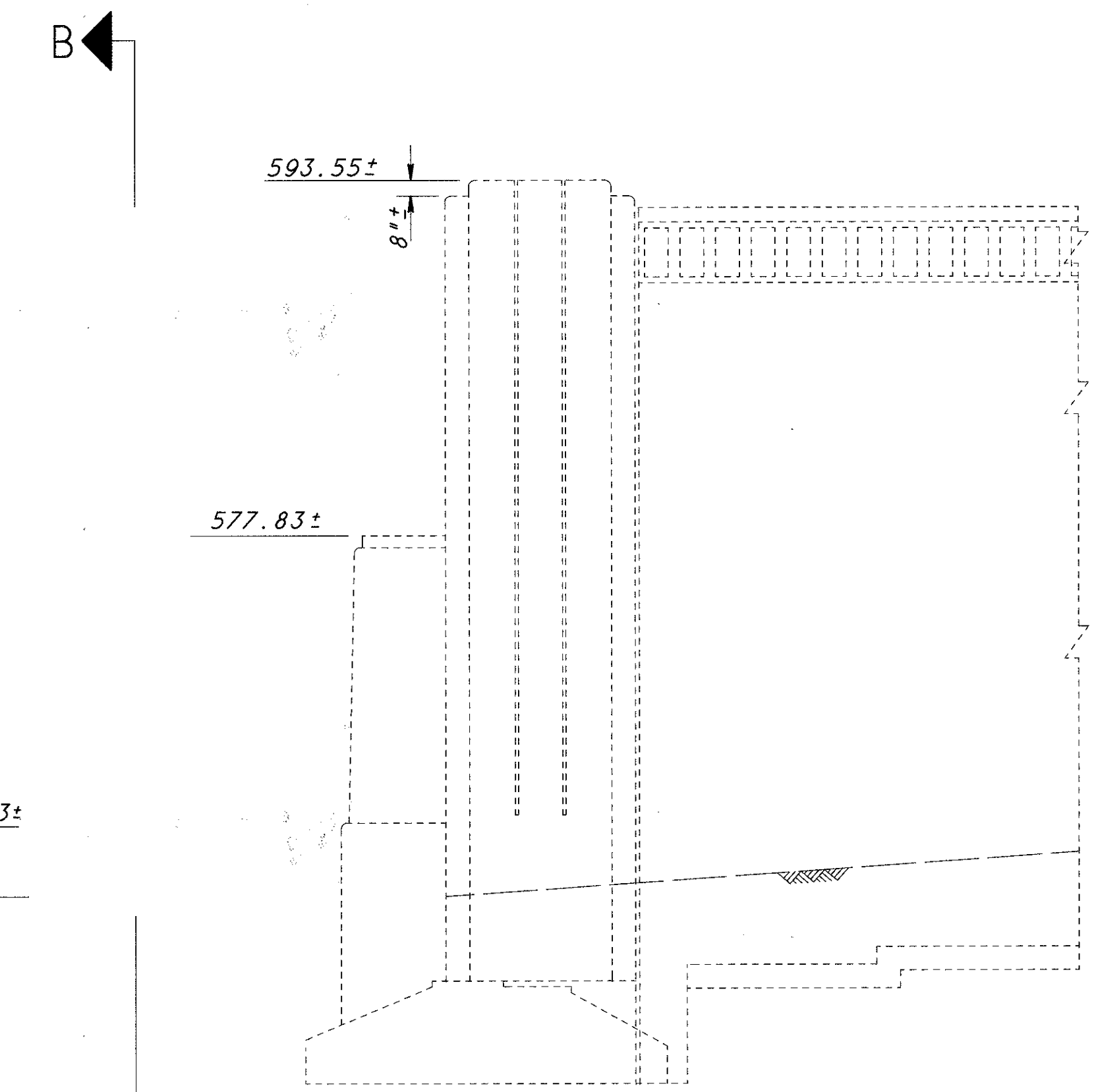
		14/108
REAR ABUTMENT MODIFICATION (ABUTMENT No. 1) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY DESIGNED: ED DRAWN: ED/PLF TRACED: WDD CHECKED: IEH REVIEWED: IEH DATE: April 96		Sta. 30+55.40 Sta. 47+16.19 REVISIONS

ABUTMENT scale 5:333

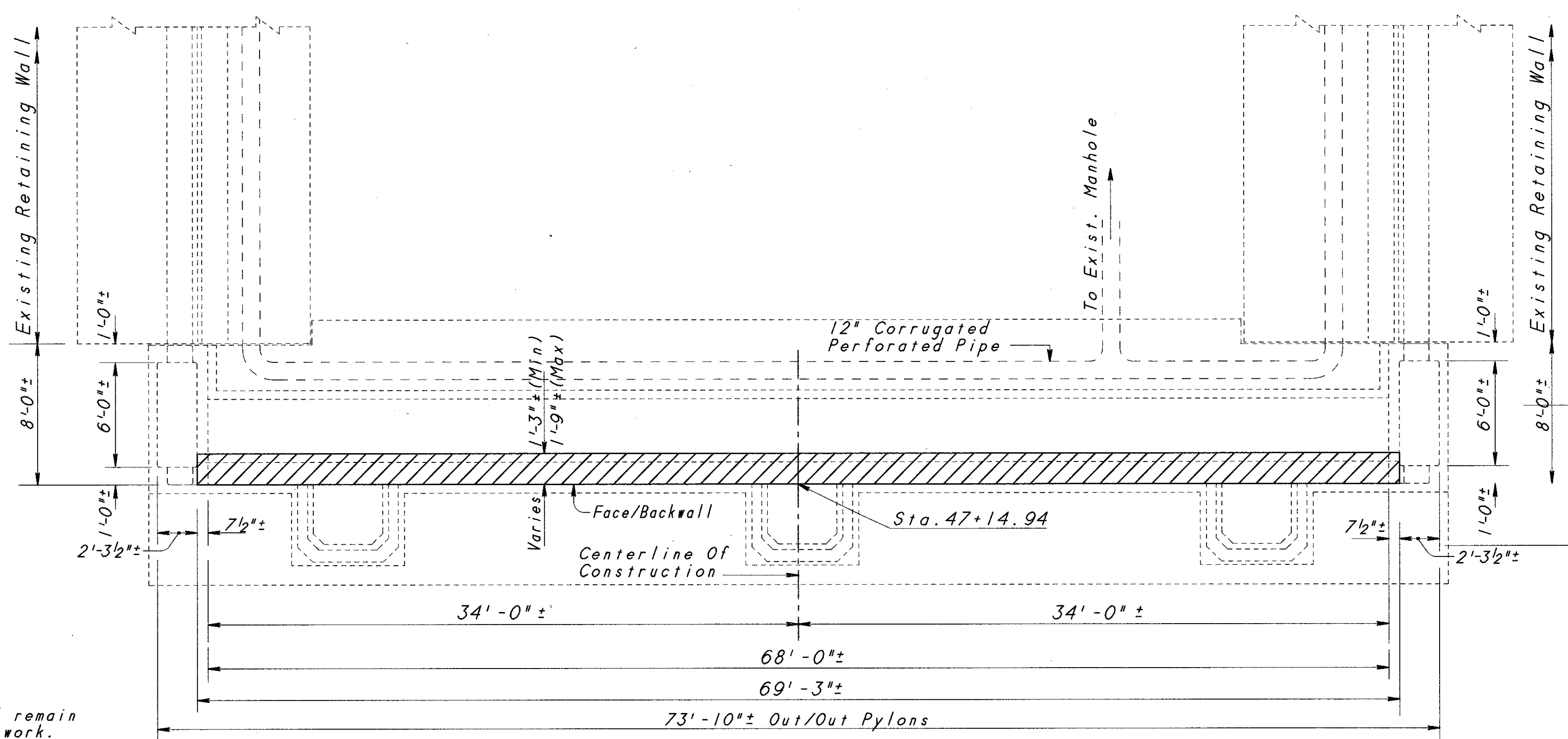


SIDE ELEVATION A-A

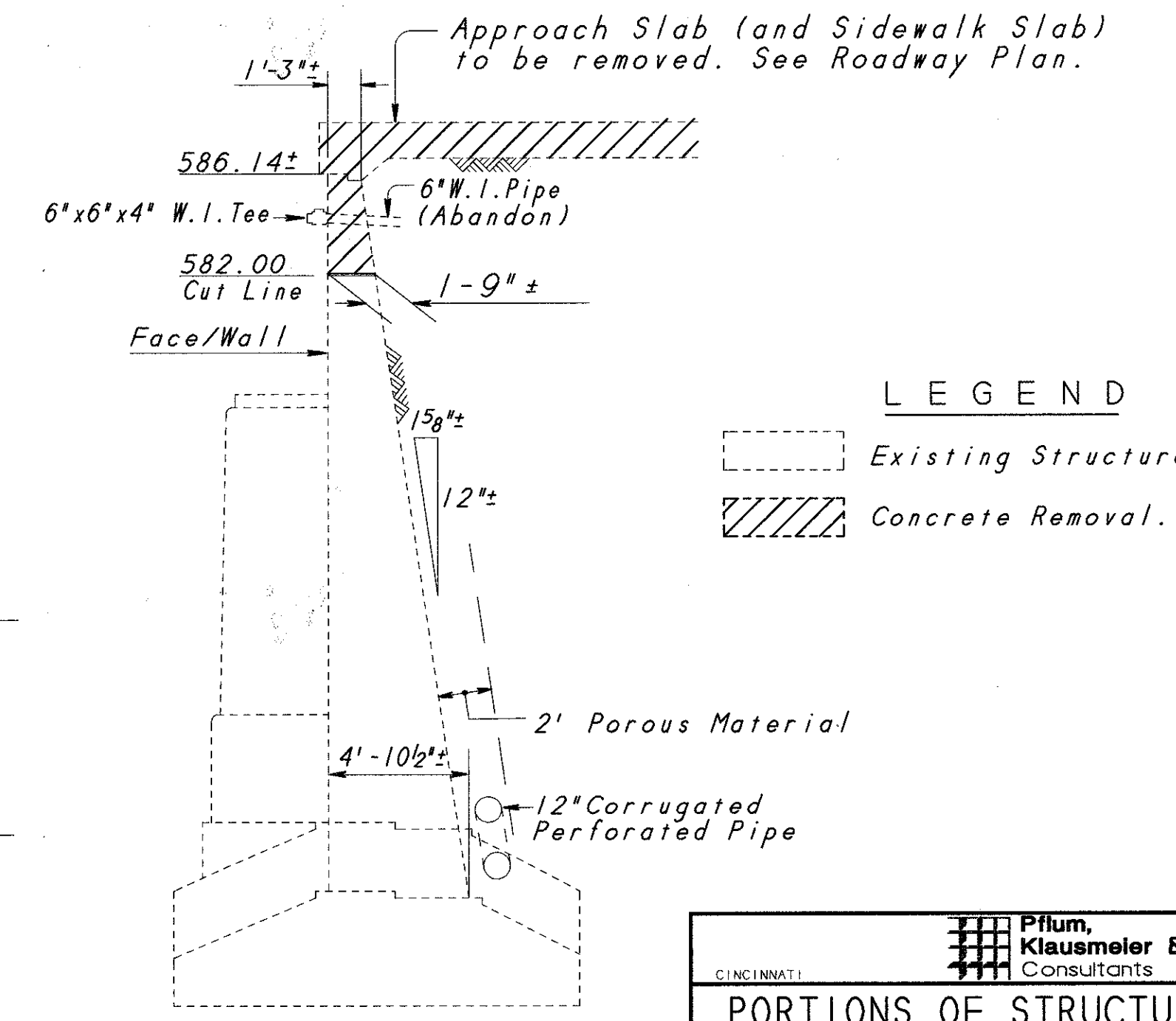
ELEVATION



SIDE ELEVATION B-B



PLAN



TYPICAL SECTION

LEGEND

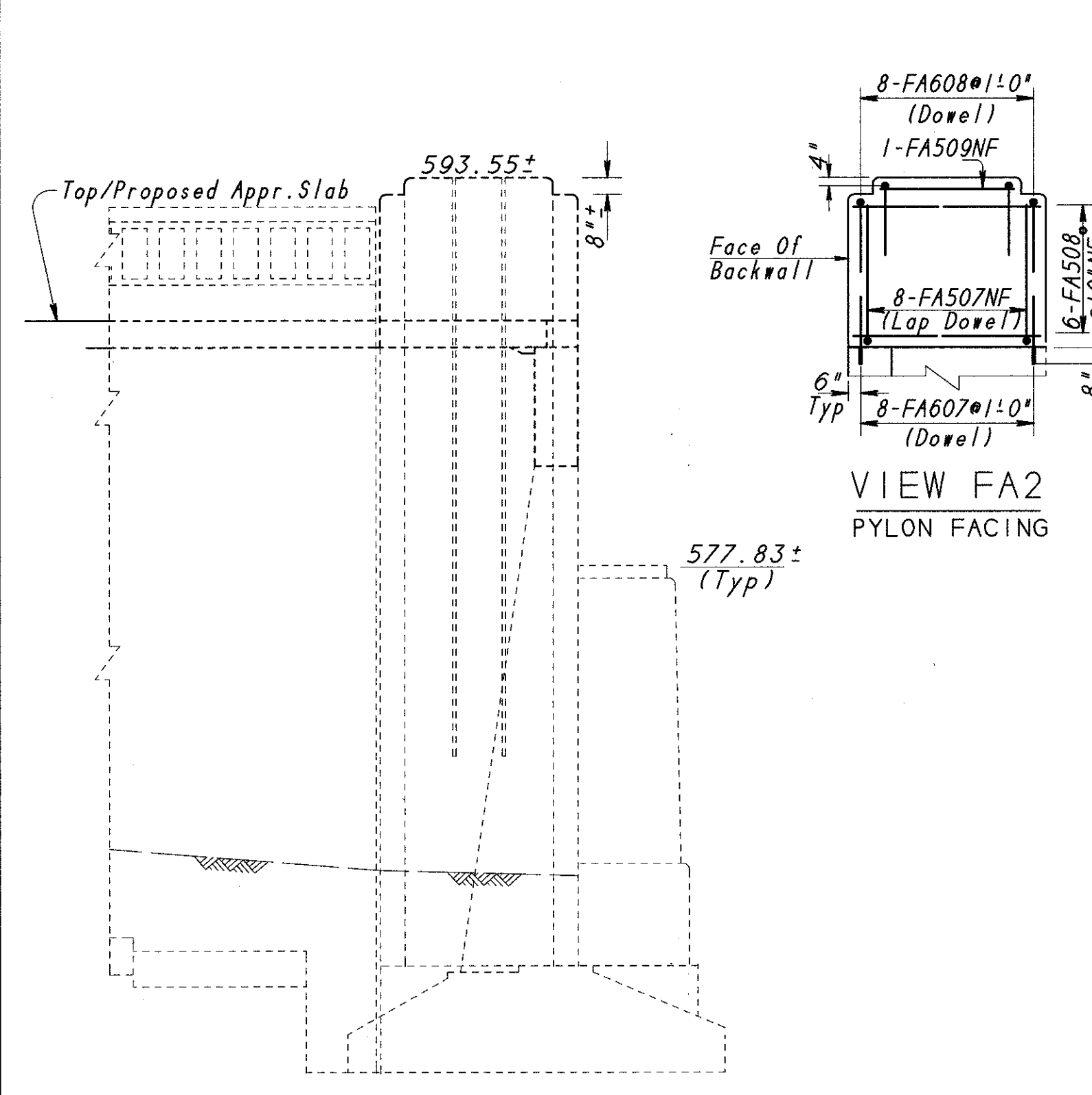
Existing Structure To Remain.

Concrete Removal.

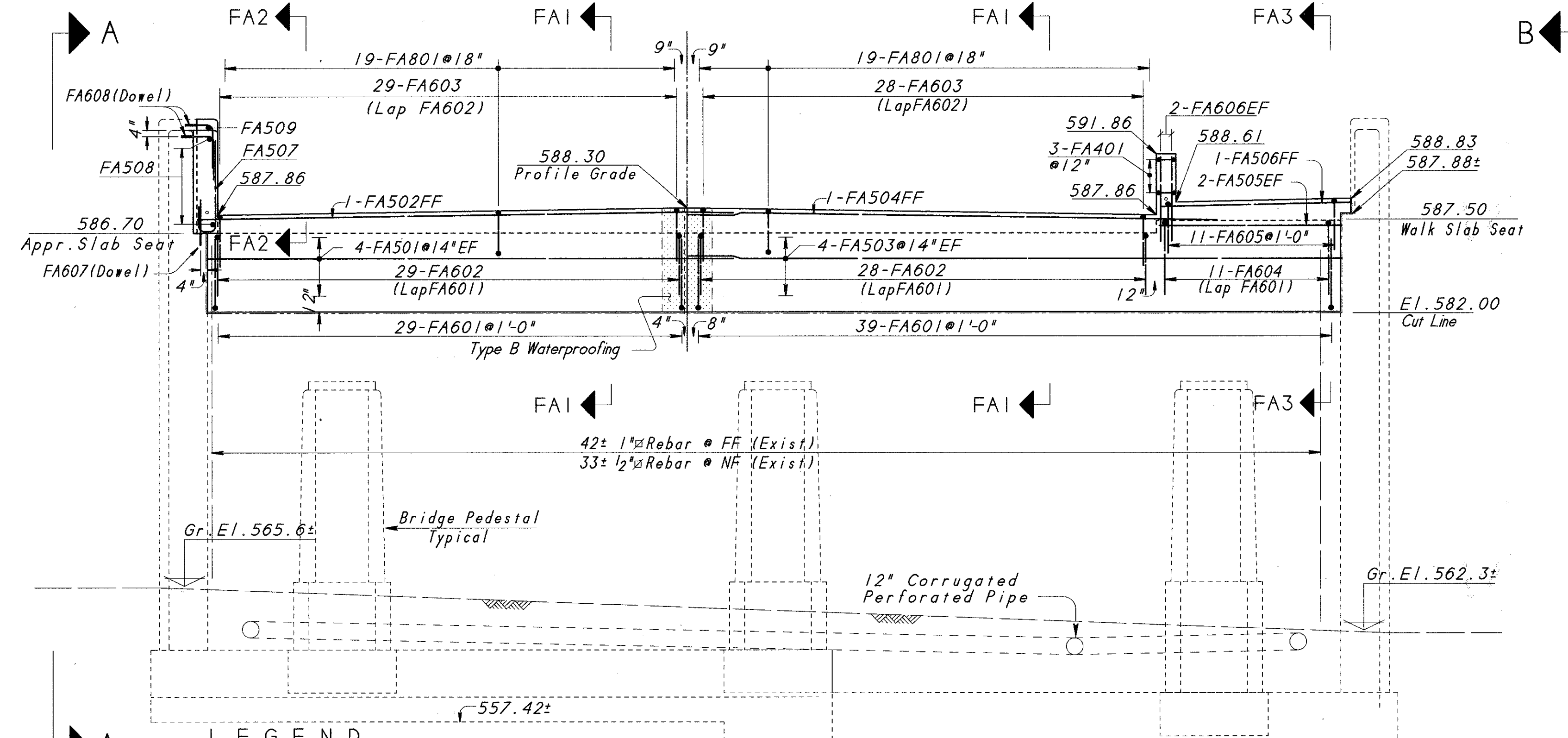
NOTES:

- 1.- Work This Sheet With SH.16/108 FORWARD ABUTMENT MODIFICATION.
- 2.- See GENERAL NOTES, SH.10/108 Concerning Construction Joint Preparation Except That At Least 3'-0" Length Of Protruding Existing Reinforcing Steel Shall Be Left In Place And Incorporated Into The New Work.
- 3.- All existing reinforcing steel in portions of the Abutment Backwall to be removed and replaced shall remain and be protected and be incorporated into the new work.
- 4.- REPLACEMENT OF EXISTING REINFORCING STEEL. - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 100 pounds is included in Item 509 for this purpose.

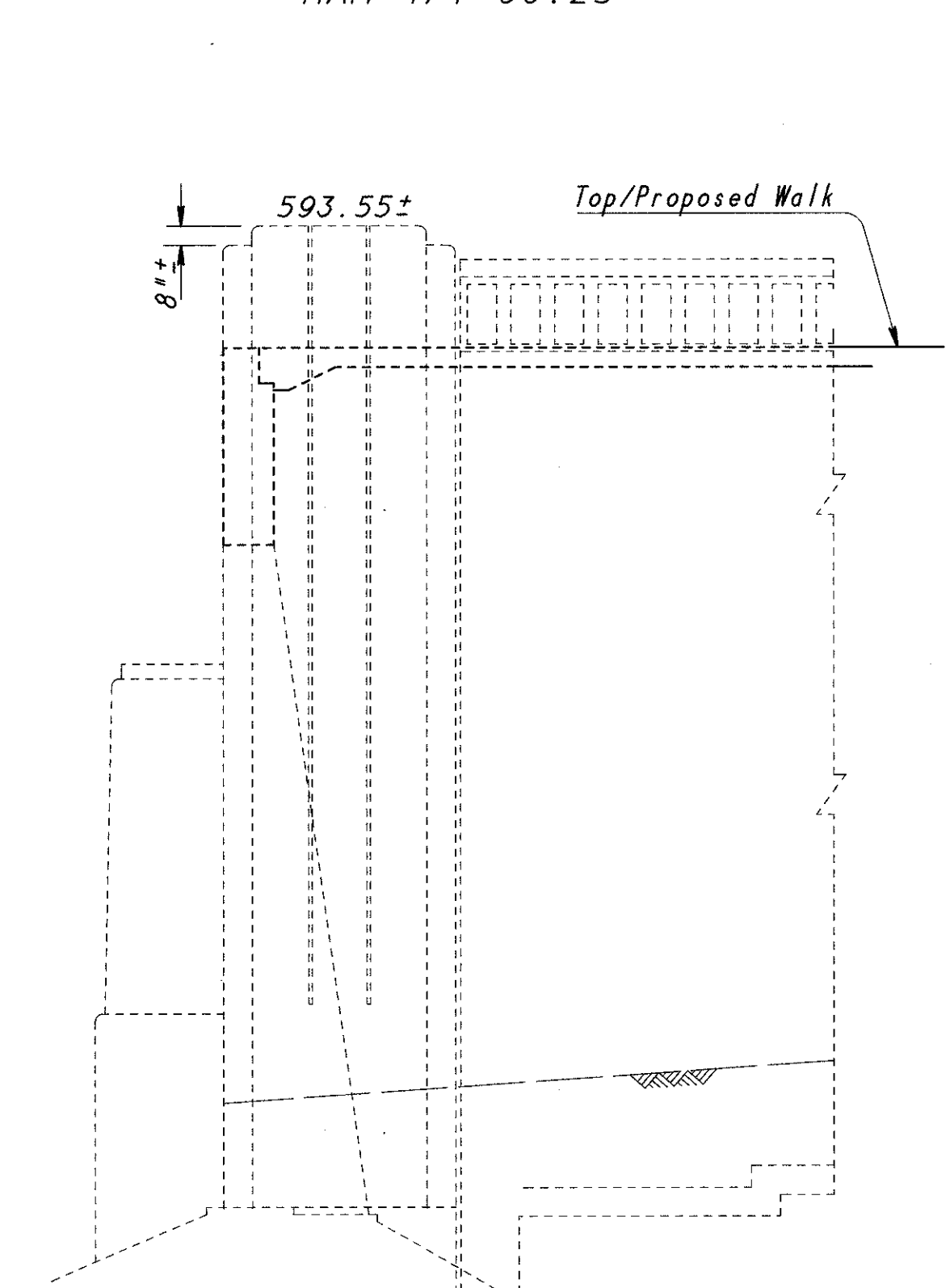
Pflum, Klausmeyer & Gehrum Consultants		15/108
PORTIONS OF STRUCTURES REMOVED FORWARD ABUTMENT (ABUTMENT No. 18)		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	CHECKED WDD
TRACED	DATE IEH April 96	REVIEWED



SIDE ELEVATION A-A



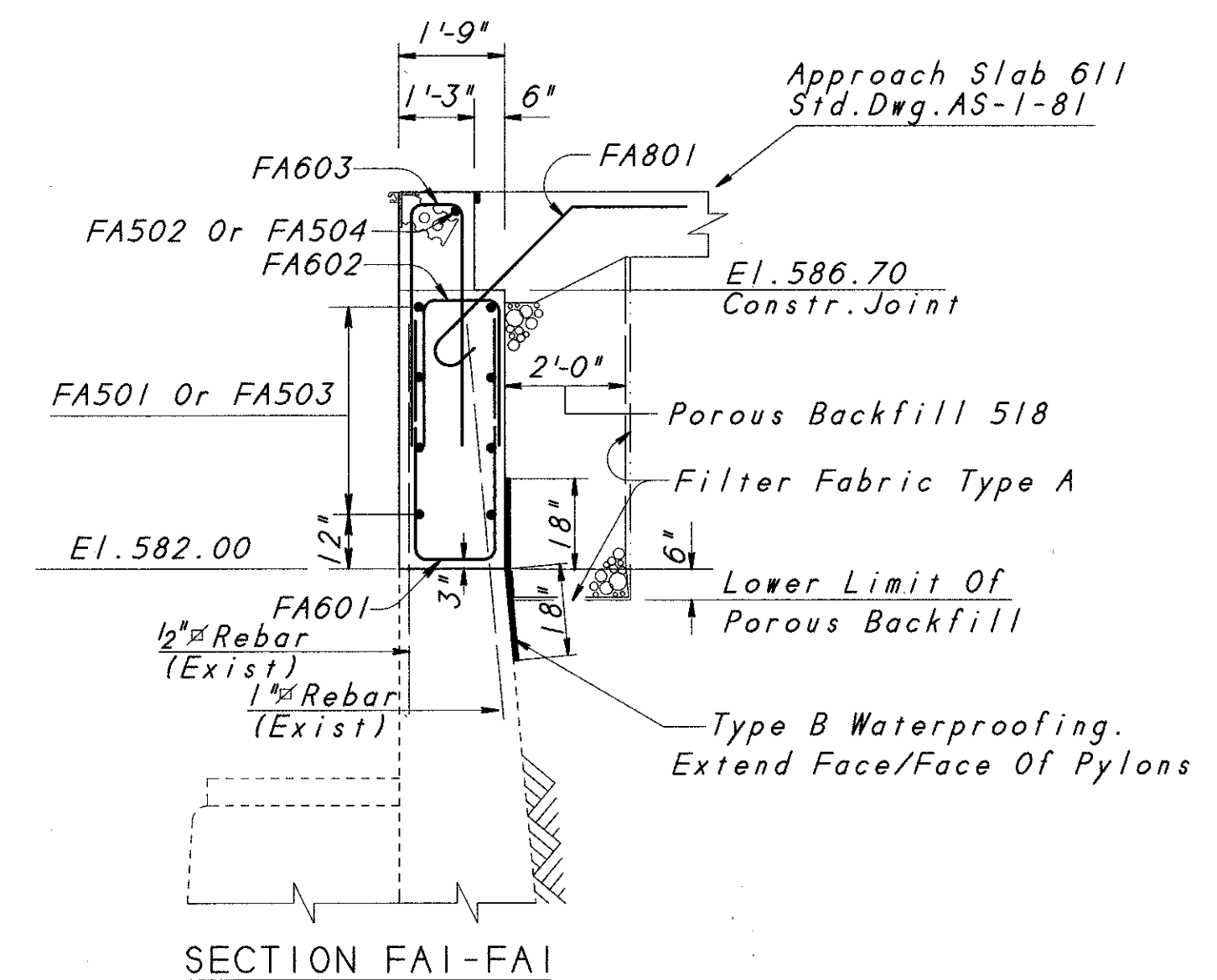
ELEVATION



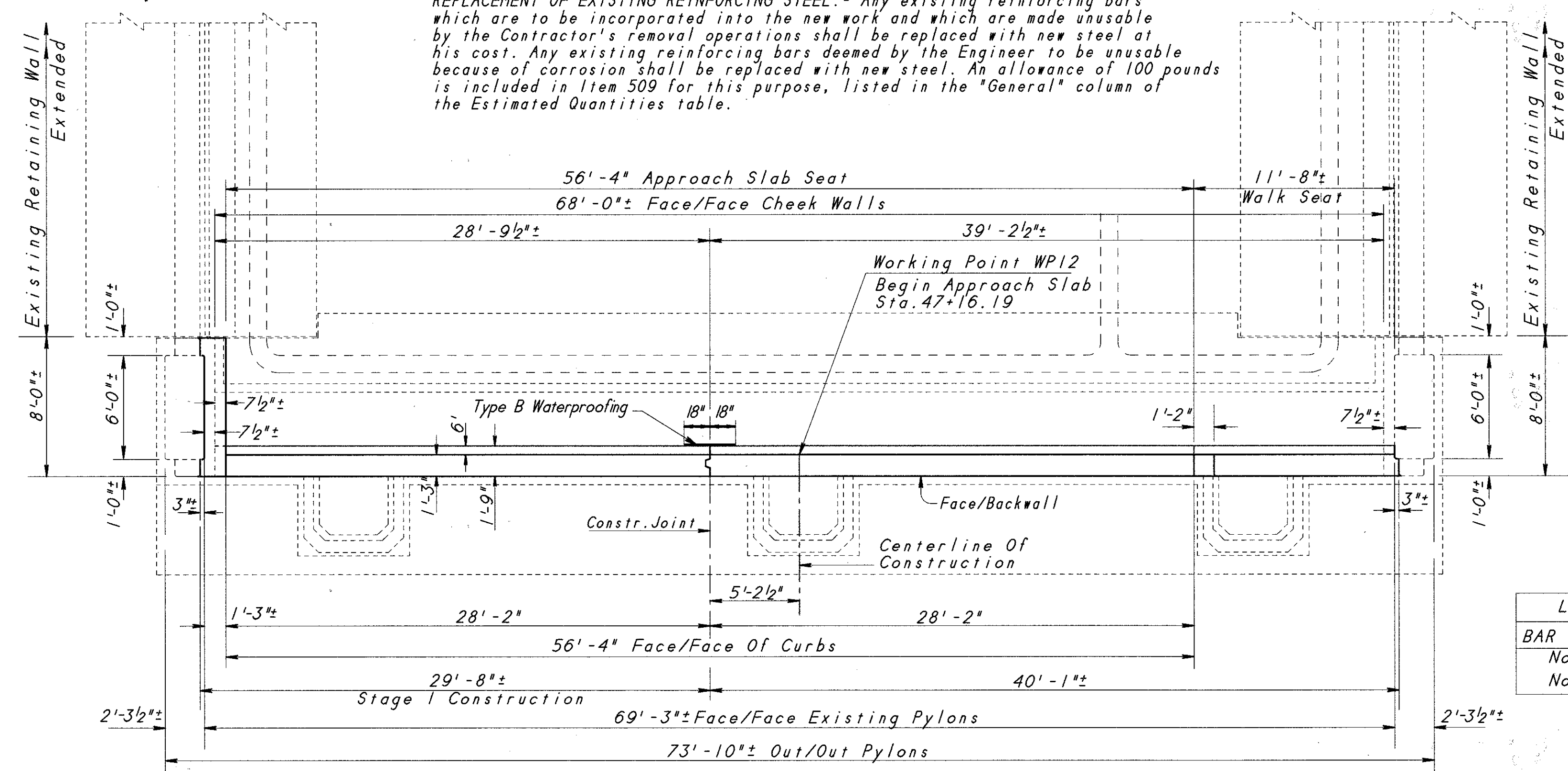
SIDE ELEVATION B-B

LEGEND
 Existing Structure
 New Work
 NF Denotes Near Face.
 FF Far Face.
 EF Each Face.
 FA Forward Abutment Rebar.

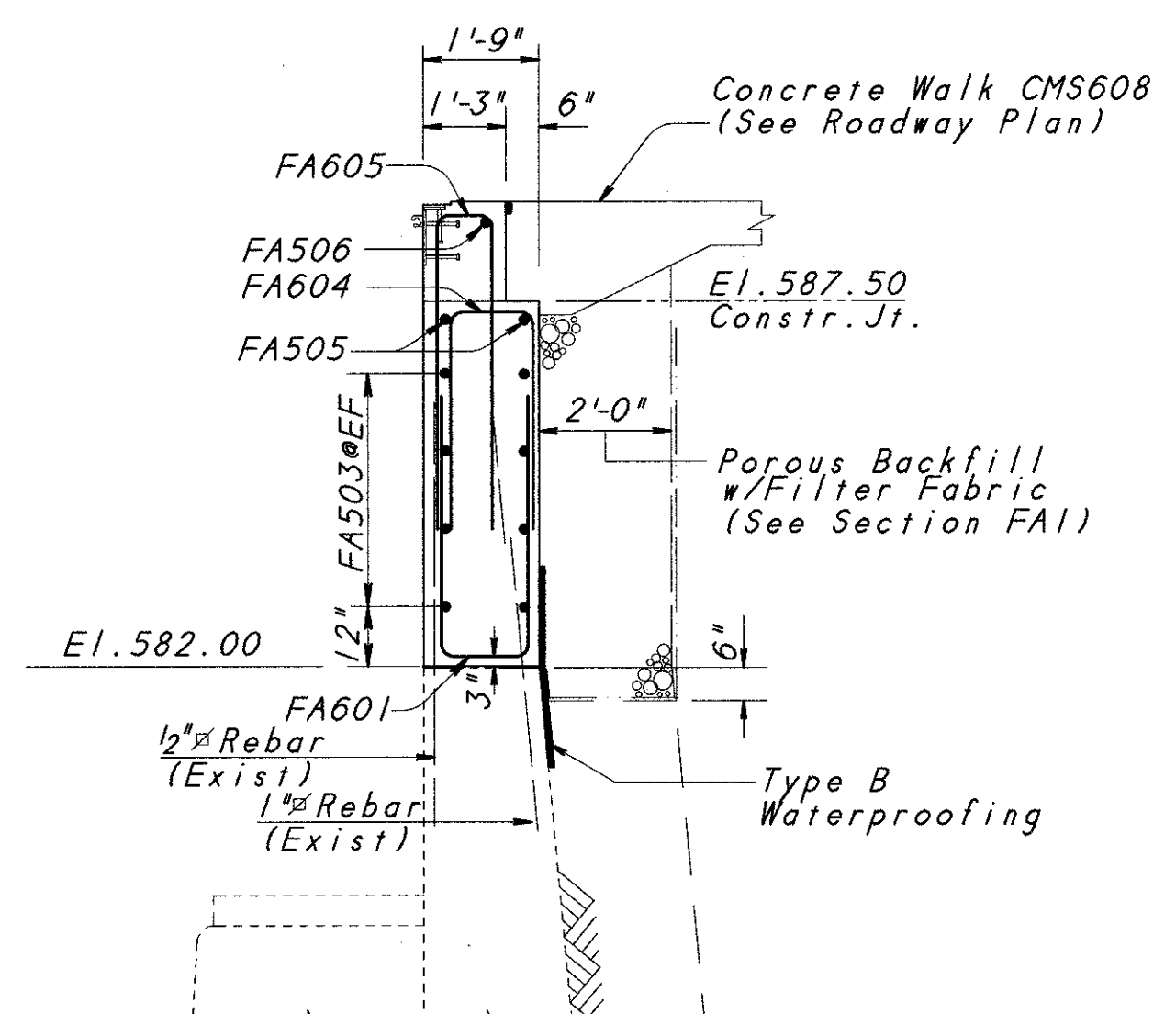
REPLACEMENT OF EXISTING REINFORCING STEEL. - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 100 pounds is included in Item 509 for this purpose, listed in the "General" column of the Estimated Quantities table.



SECTION FA1-FA1



P L A N



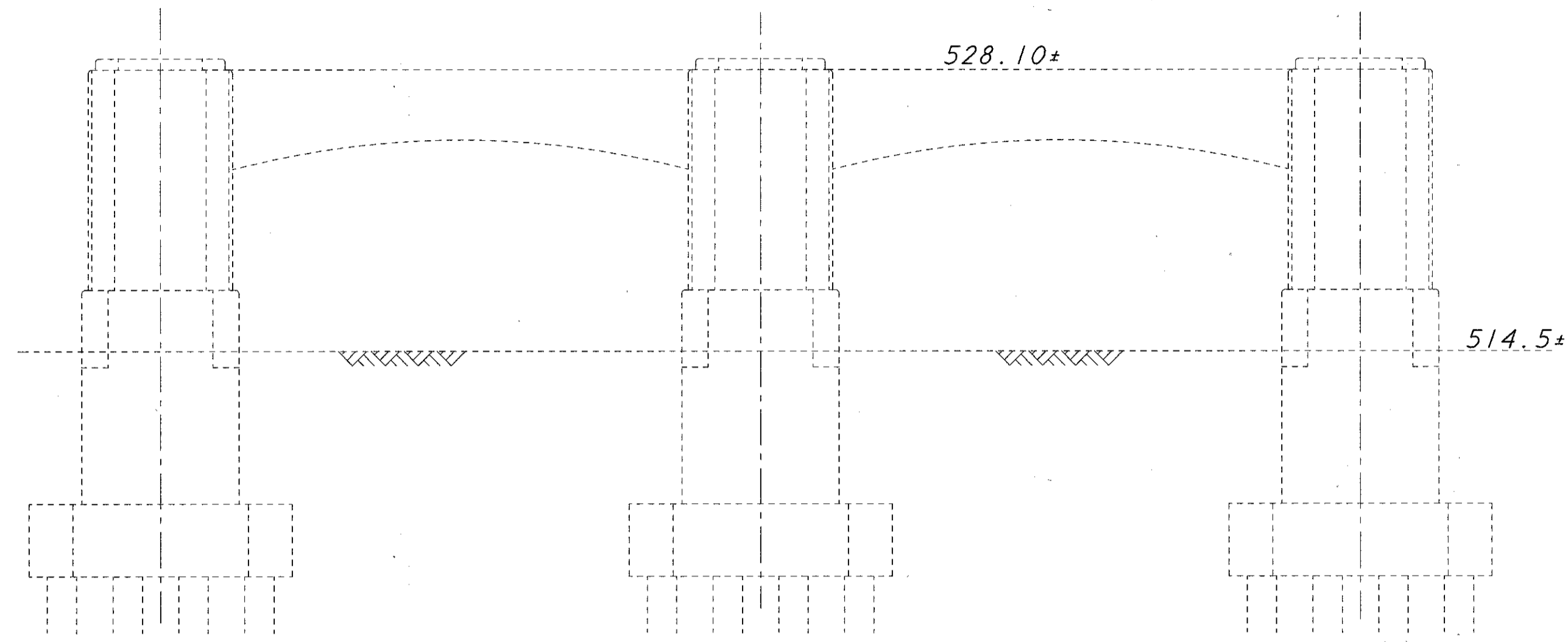
SECTION FA3-FA3

- NOTES:
- See GENERAL NOTES Sh. 10/108. On Construction Joint Preparation At Least 3'-0" Length Of Protruding Existing Reinforcing Steel Shall Be Left In Place And Incorporated Into The New Work.
 - Work This Sheet With Sh. 15/108 PORTIONS OF STRUCTURES REMOVED, FORWARD ABUTMENT.
 - See REINFORCING STEEL BAR LIST, Sh. 86/108
 - Clearance Between Reinforcing Steel And Surface Of Concrete Shall Be As Per CMS509 Unless Noted Otherwise.

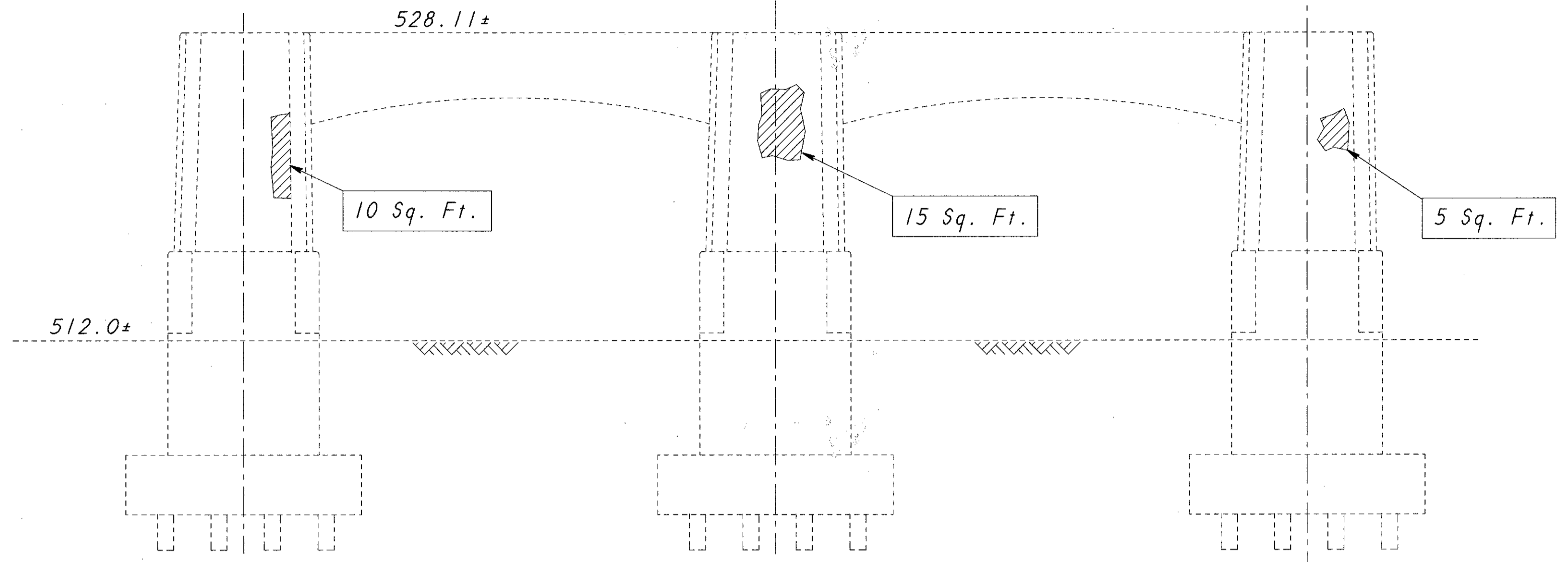
LAP LENGTH TABLE	
BAR SIZE	LAP LENGTH
No. 5	2'-9"
No. 6	3'-4"

Pflum, Klausmeier & Gehrum Consultants		16/108
CINCINNATI OHIO		
FORWARD ABUTMENT MODIFICATION (ABUTMENT No. 18)		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

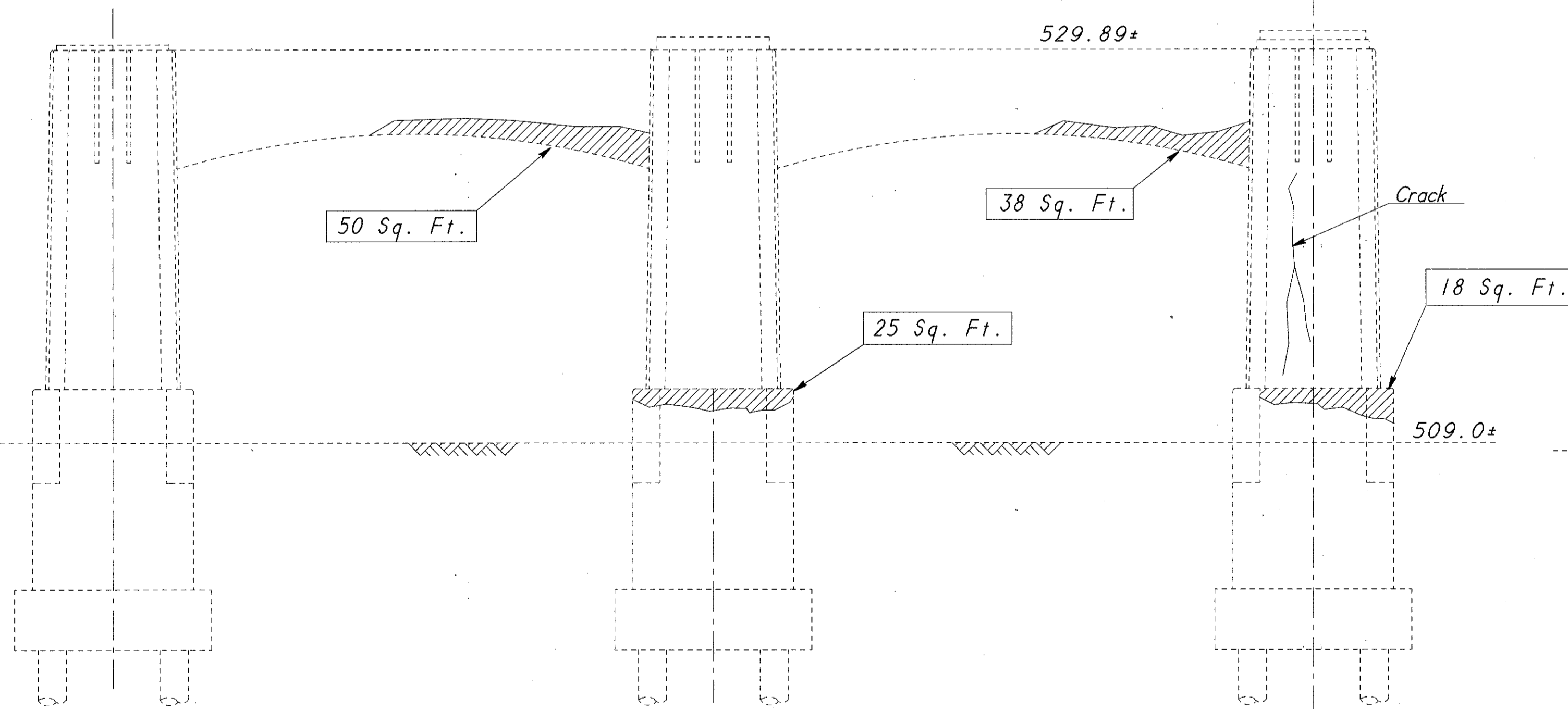
File: ABUTRM Scale 5:333



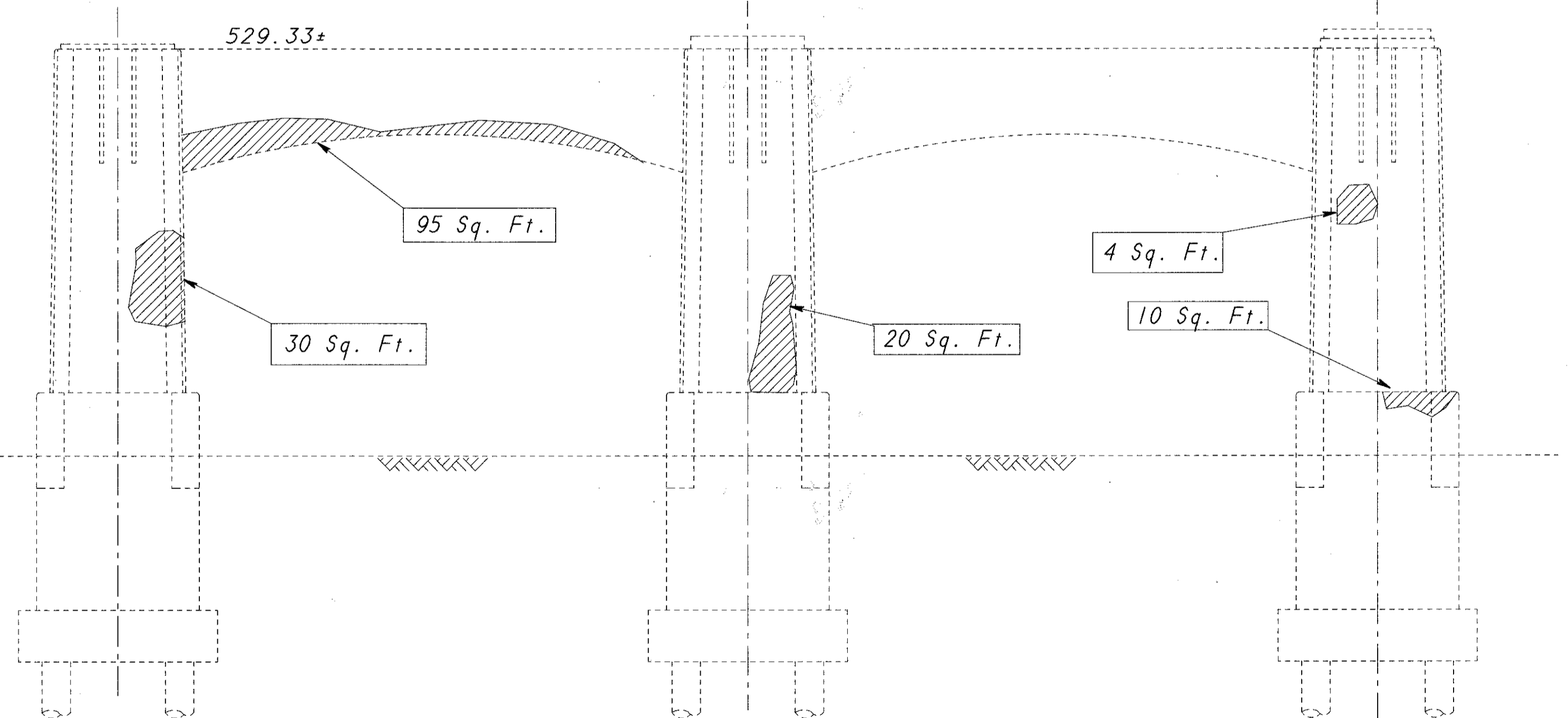
PIER No. 2



PIER No. 3



PIER No. 4



PIER No. 5

LEGEND

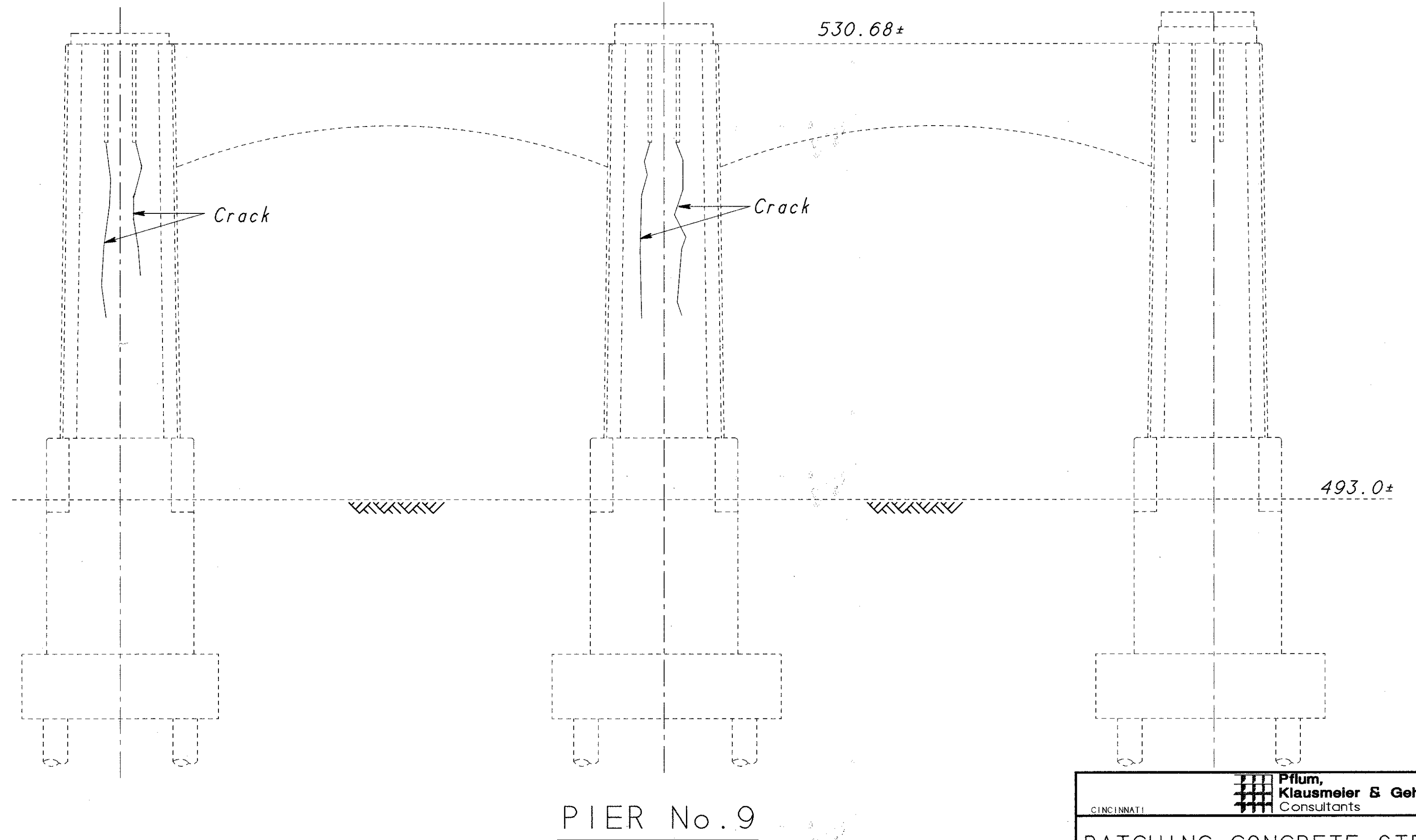
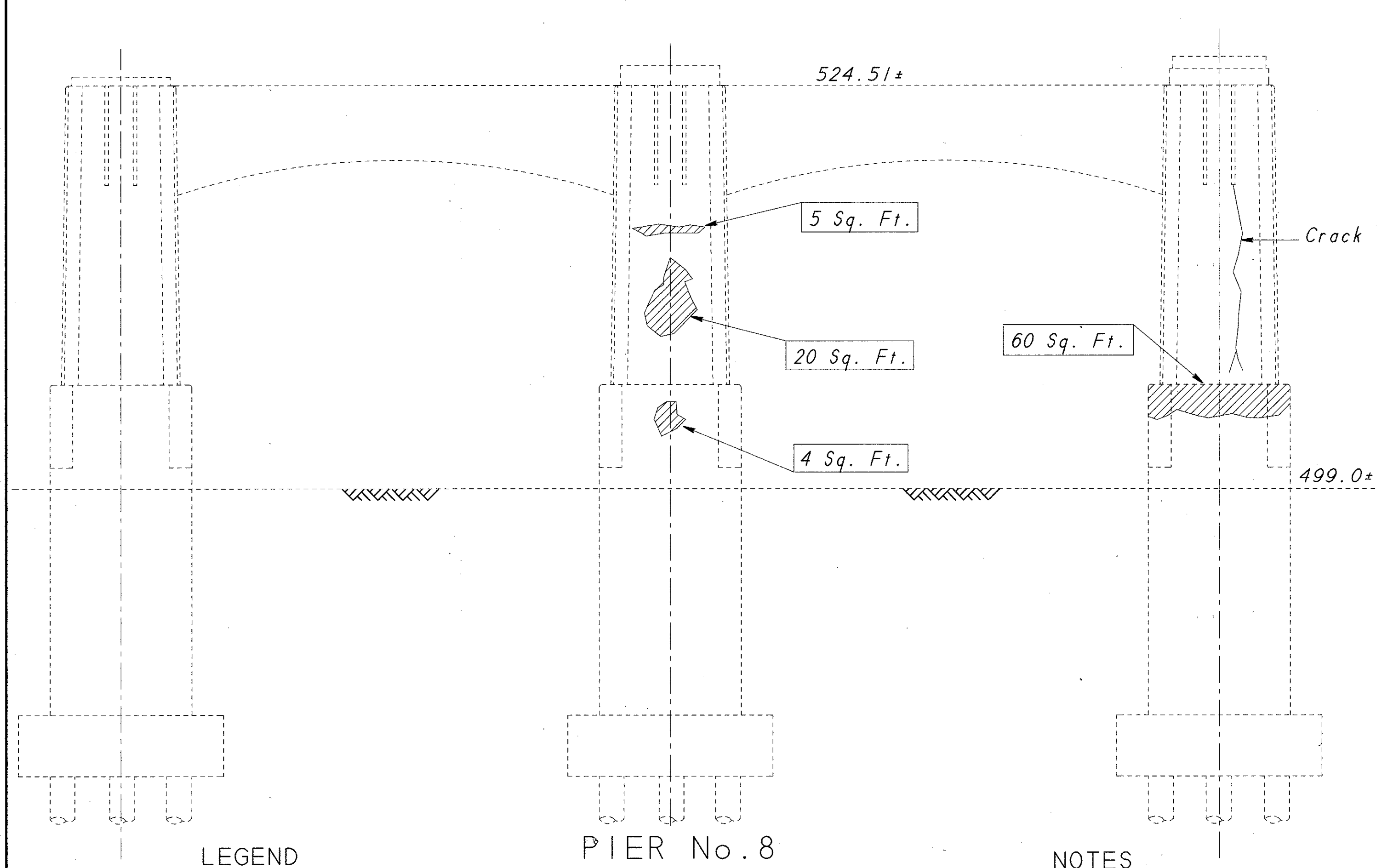
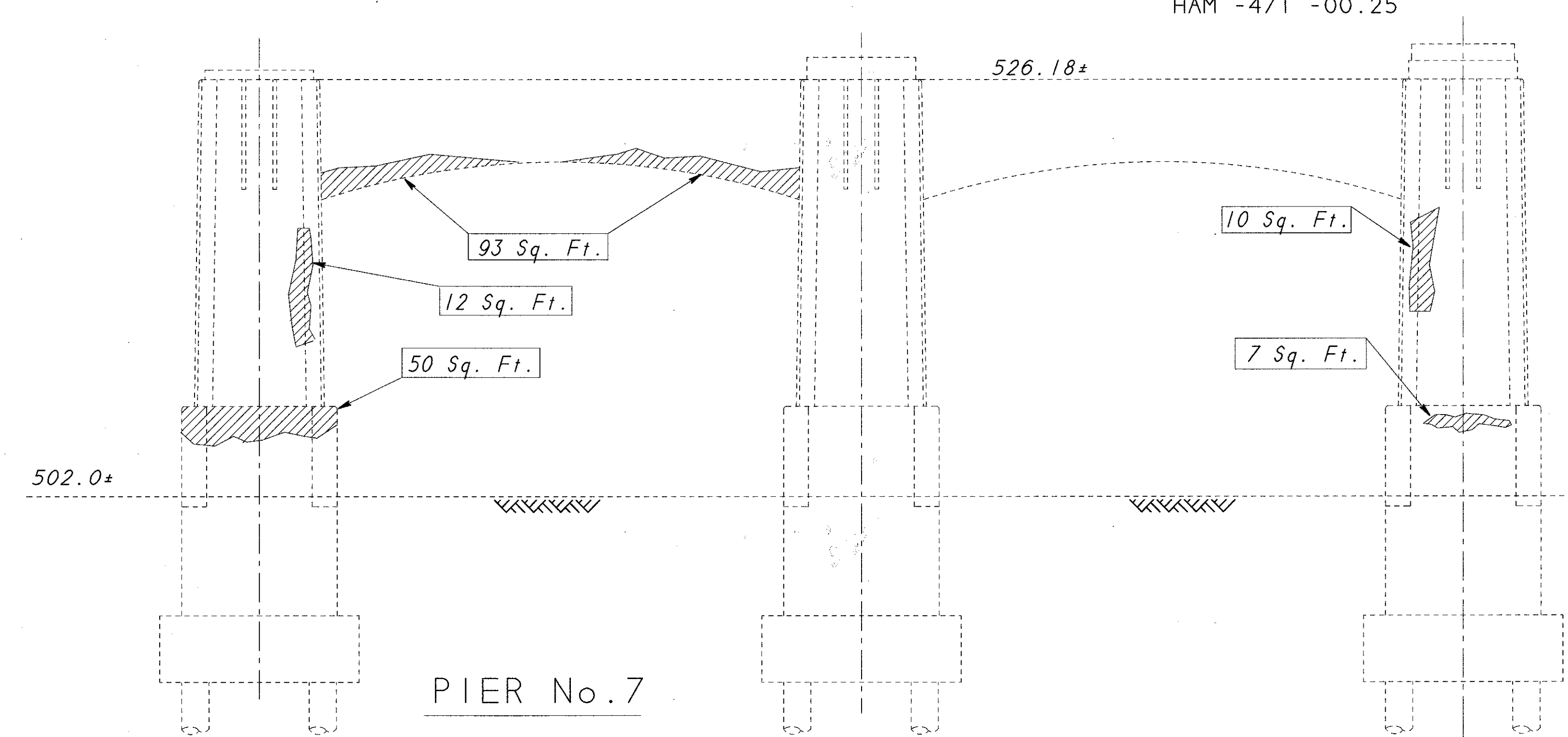
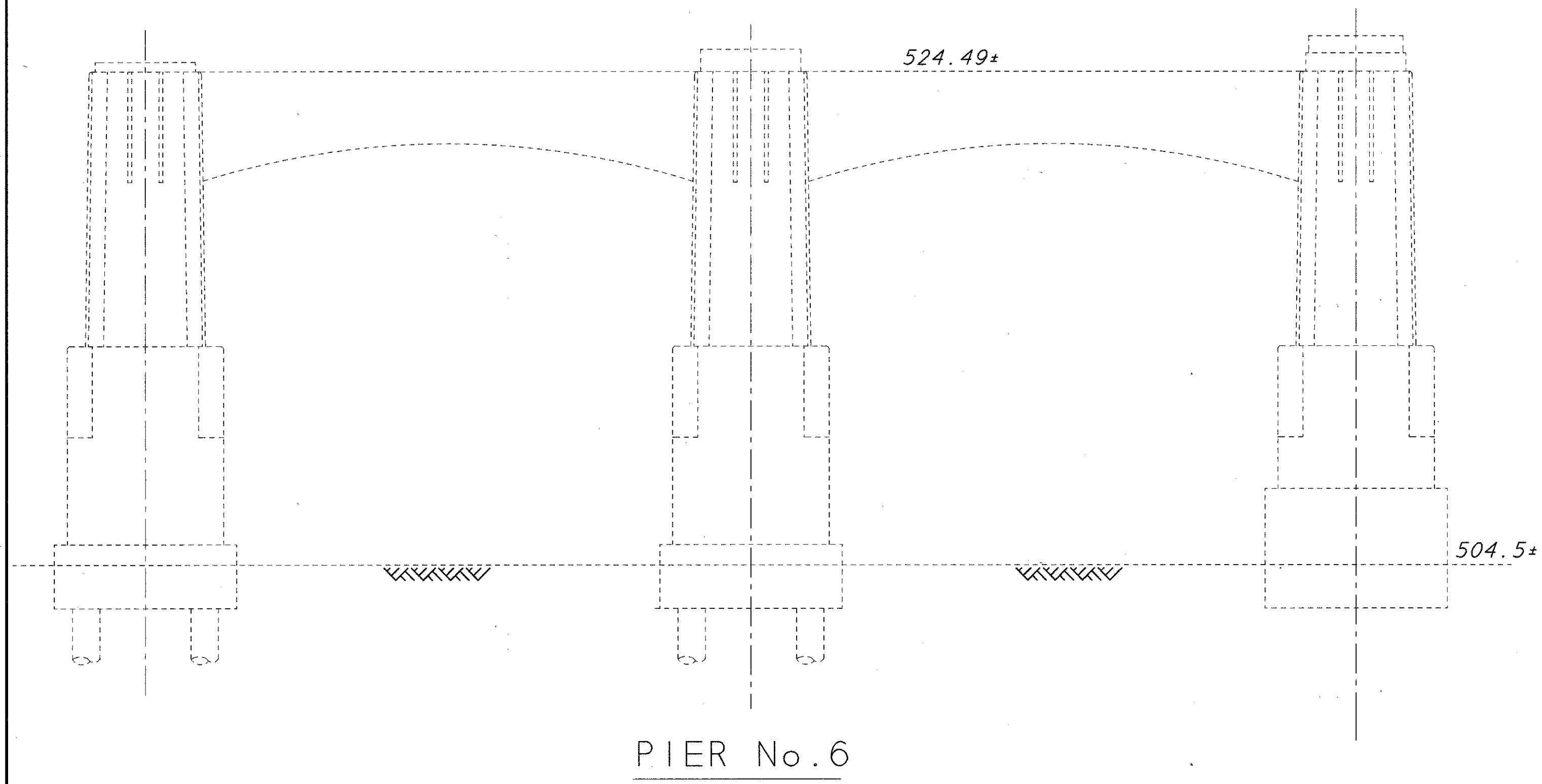
- Denotes Existing Structures
- Item Special - Patching Concrete Structures With Trowelable Mortar.
(Total This Sh. 320 Sq. Ft.)
- Item Special - Concrete Repair By Epoxy Injection, Including Surface Preparation.
(Total this sheet: 14 Lin.Ft.)

NOTES . -


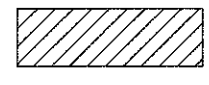
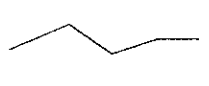
- 1.-Area of patching shown on pier elevation represents the total patching area in pier.
- 2.-Representation of the existing structure is approximate.

Pflum, Klausmeier & Gehrm		17/108
CINCINNATI, OHIO Consultants		
PATCHING CONCRETE STRUCTURES PIERS		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
Sta. 30+55.40		
Sta. 47+16.19		
HAMILTON COUNTY		
DESIGNED ED	DRAWN ED/JDR	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96
		REVISED

PIERSH Scale 5:3333



LEGEND

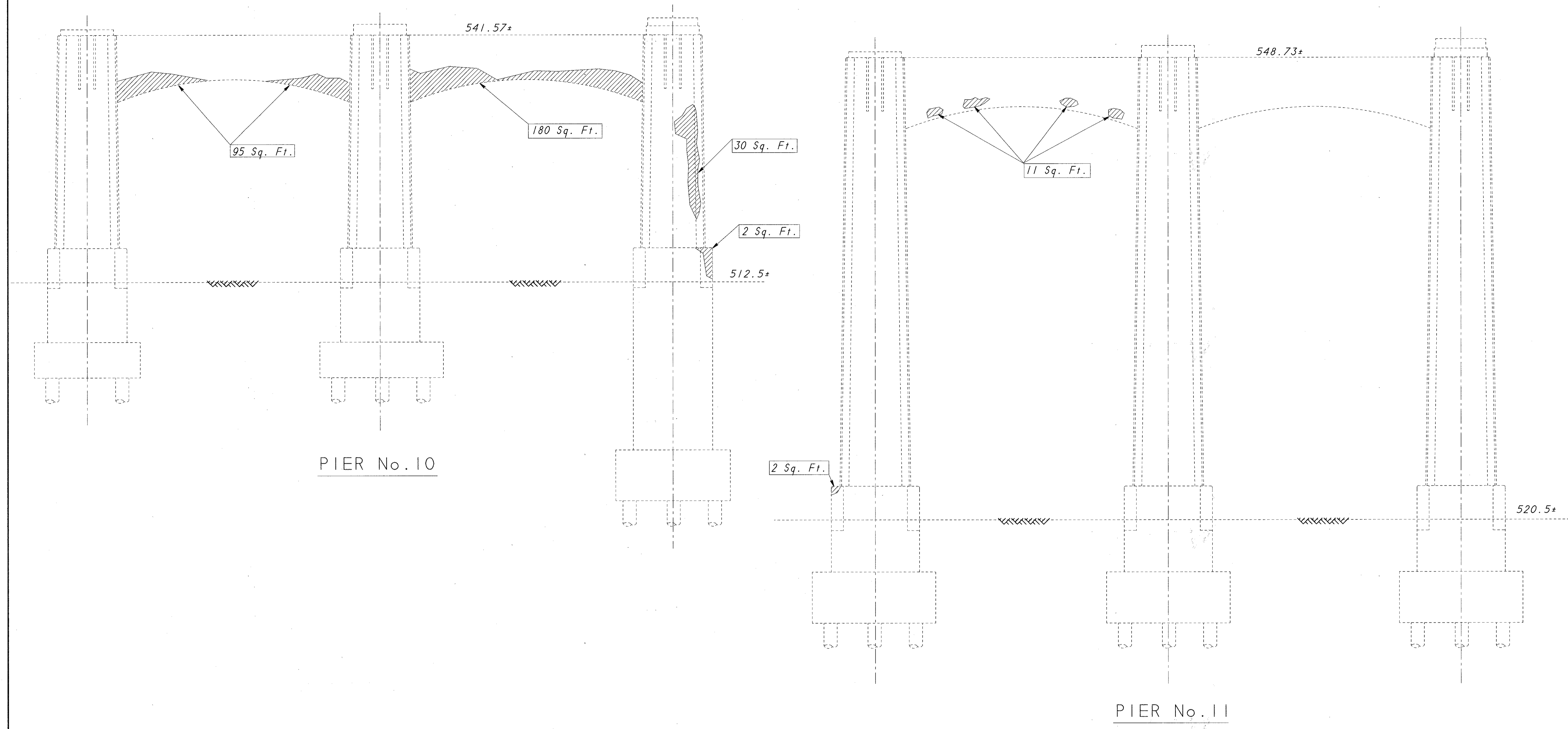
-  Denotes Existing Structures
-  Item Special - Patching Concrete Structures With Trowelable Mortar. (Total This Sh. 261 Sq. Ft.)
-  Item Special - Concrete Repair By Epoxy Injection, Including Surface Preparation. (Total this sheet: 48 Lin.Ft.)

NOTES

- 1.-Area of patching shown on pier elevation represents the total patching area in pier.
- 2.-Representation of the existing structure is approximate.

Plum, Klausmeier & Gehrum		18/108
Consultants		OHIO
PATCHING CONCRETE STRUCTURES PIERS		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96
REVISED		

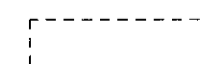

PIER No. 2 Scale 5/323




NOTES

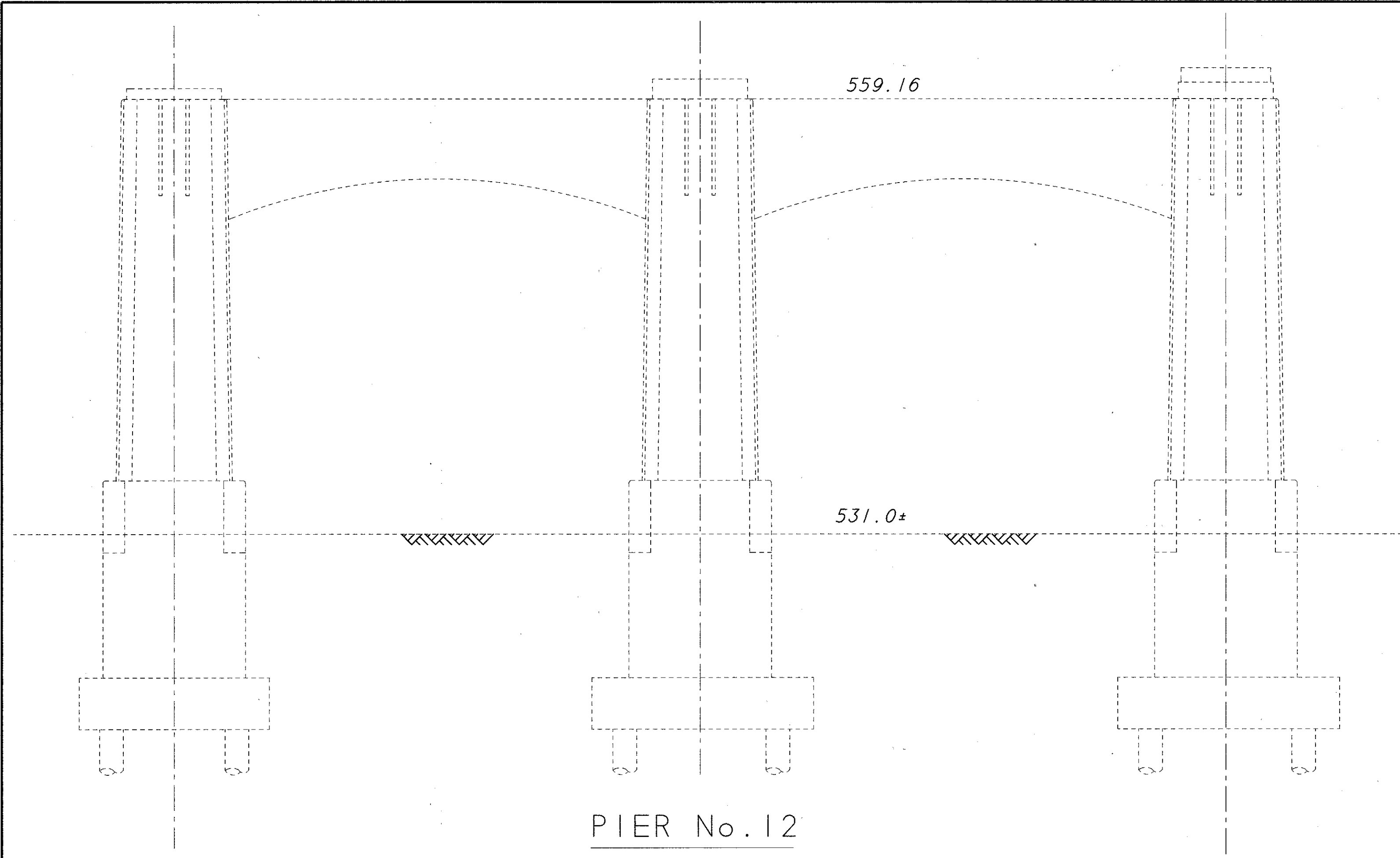
- 1.-Area of patching shown on pier elevation represents the total patching area in pier.
- 2.-Representation of the existing structure is approximate.

LEGEND

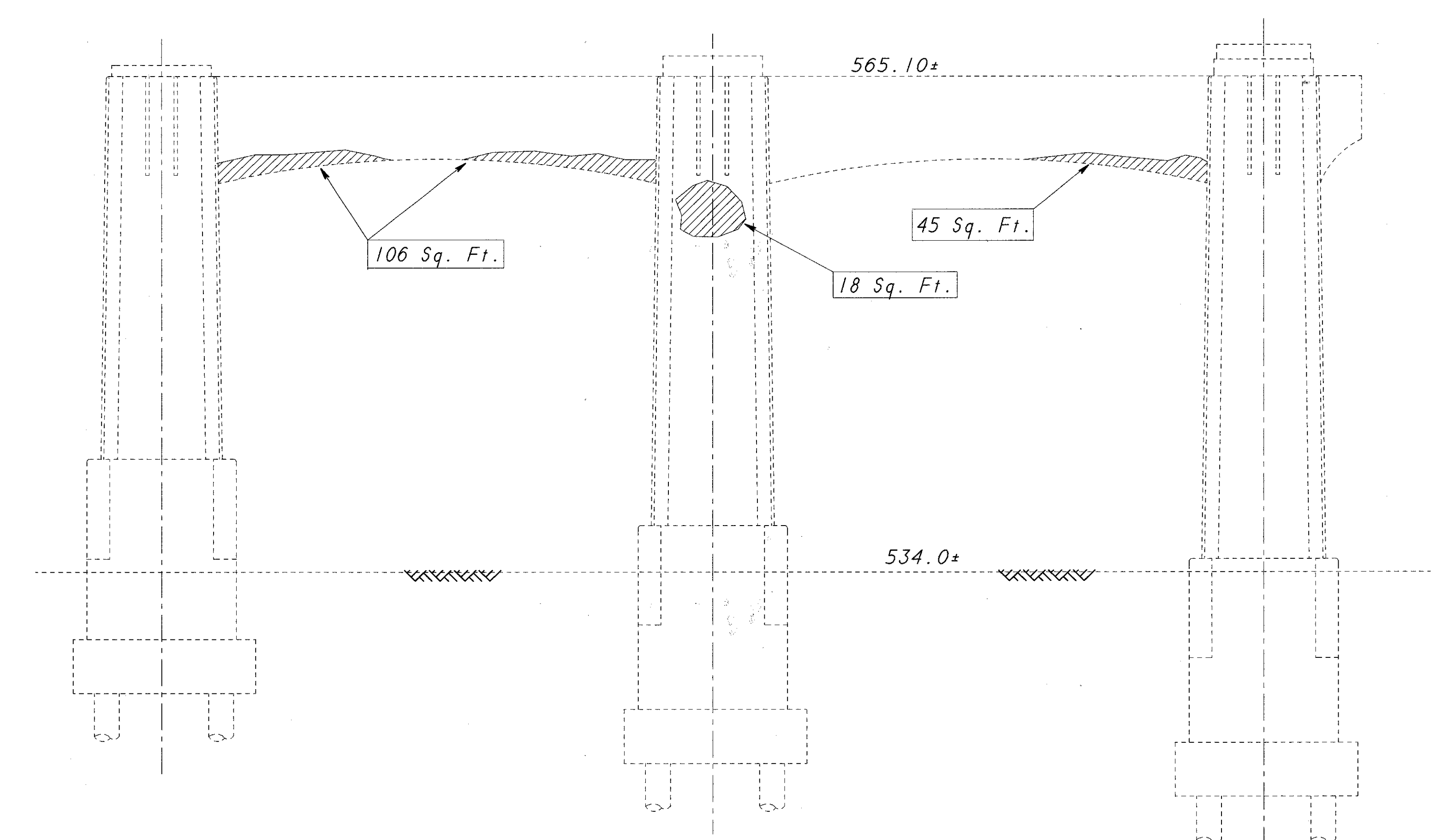
-  Denotes Existing Structures
-  Item Special -Patching Concrete Structures With Trowelable Mortar. (Total This Sh. 320 Sq. Ft.)

CINCINNATI		 Pfium, Klausmeier & Gehrm Consultants		19/108	
PATCHING CONCRETE STRUCTURES PIERS					
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)					
HAMILTON COUNTY			Sta. 30+55.40	Sta. 47+16.19	
DESIGNED ED	DRAWN JDR	TRACED WDD	CHECKED IEH	REVIEWED April 96	DATE REVISED

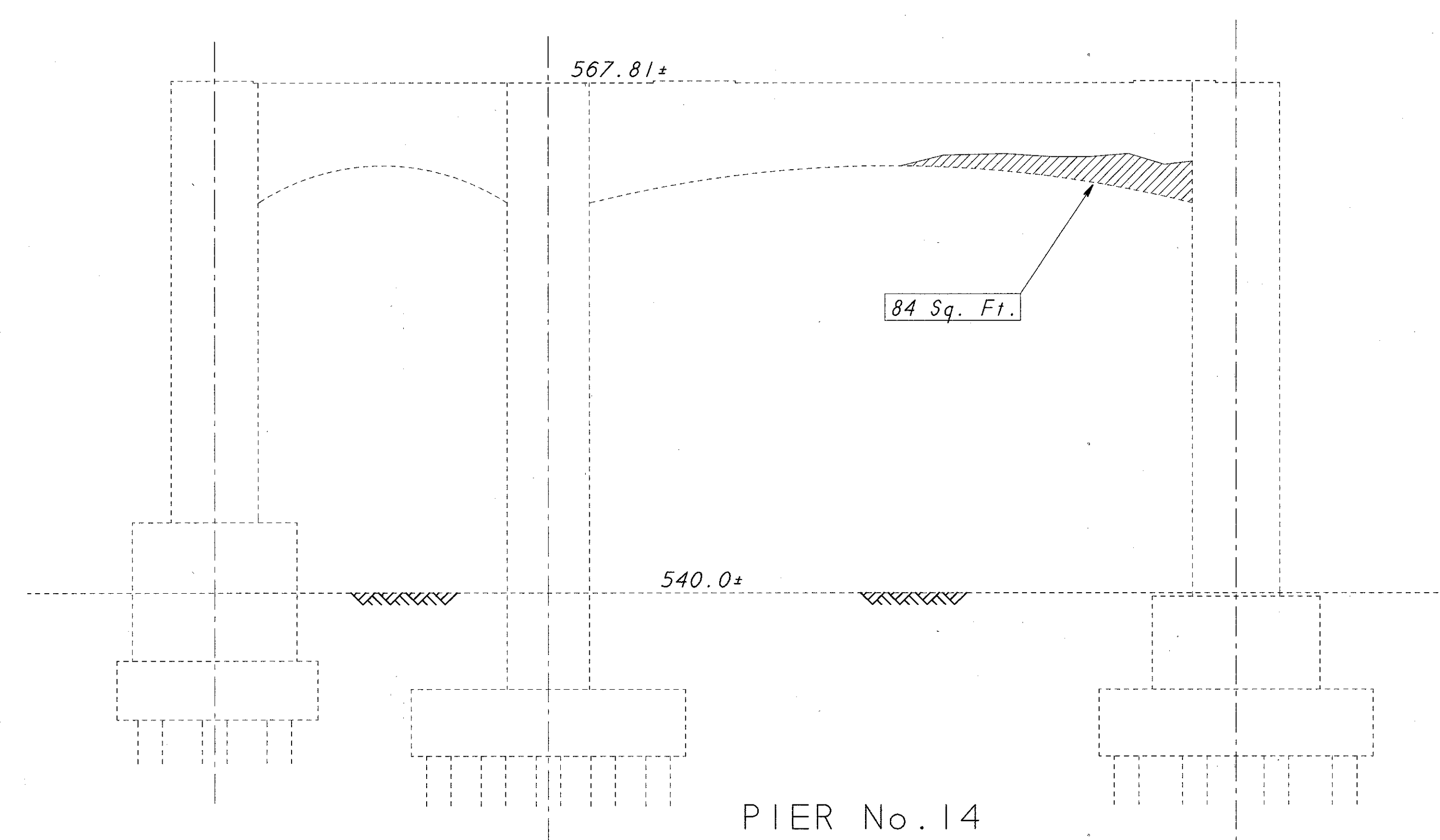
PIERS/13 Scale 5:333



PIER No. 12



PIER No. 13



PIER No. 14

LEGEND

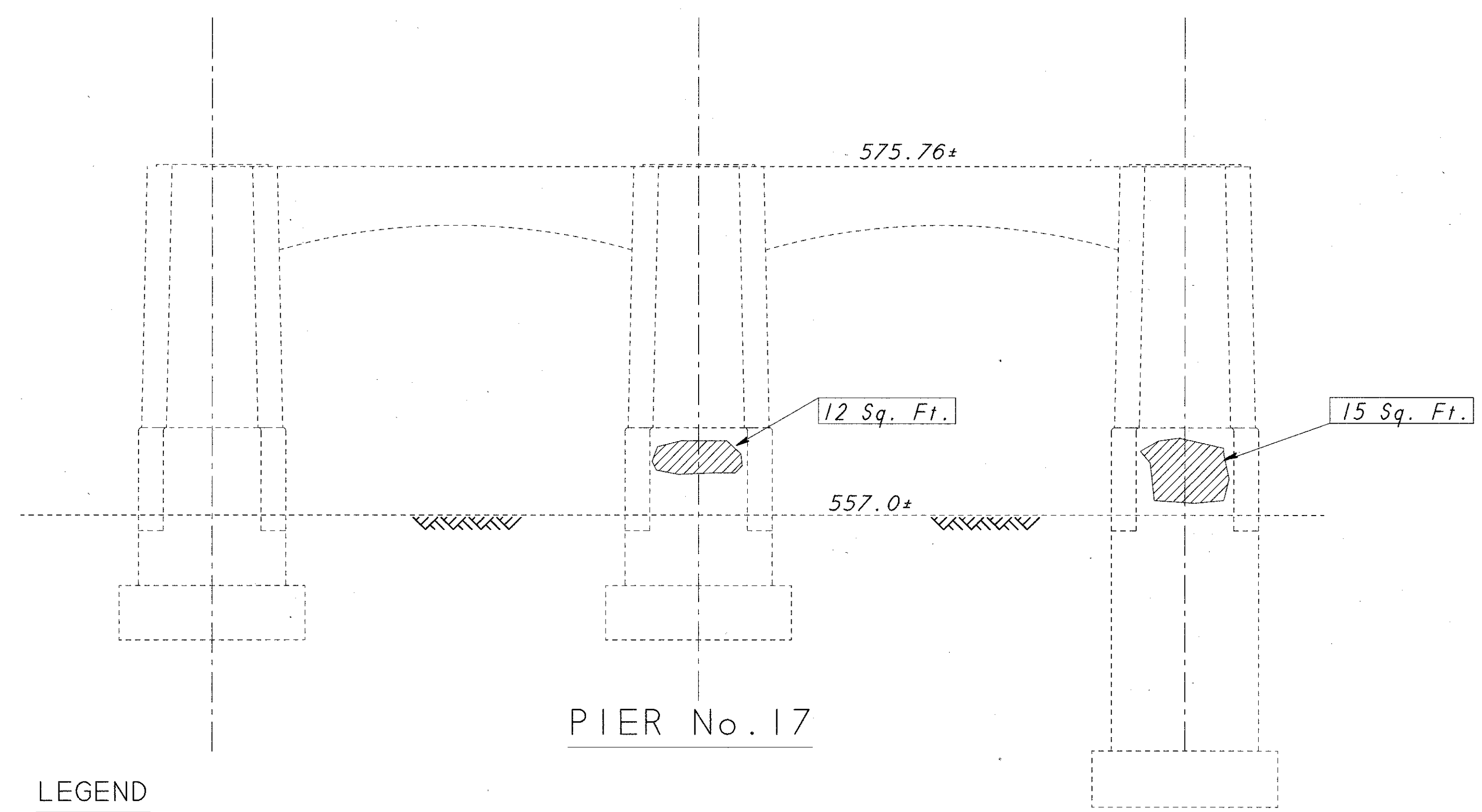
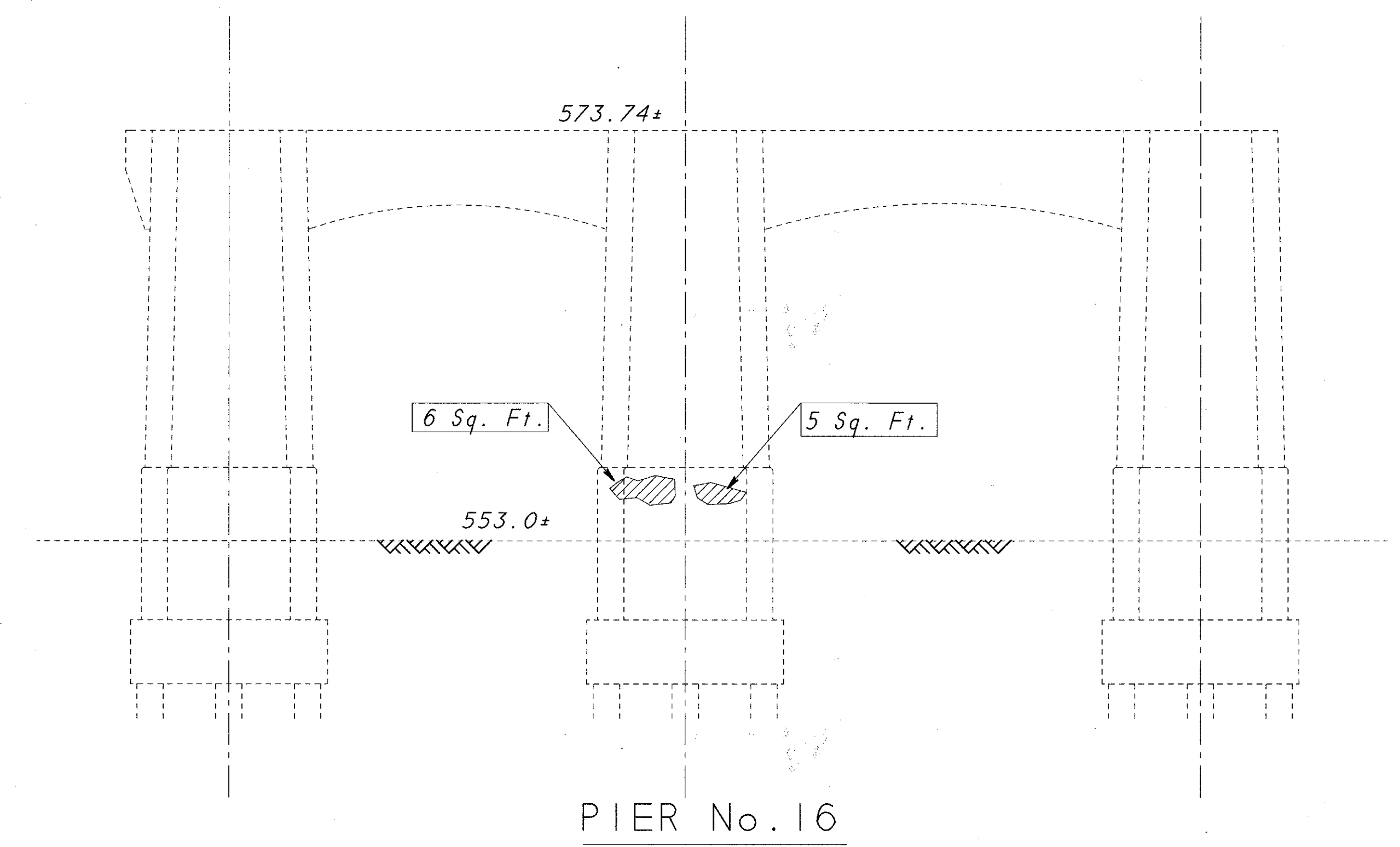
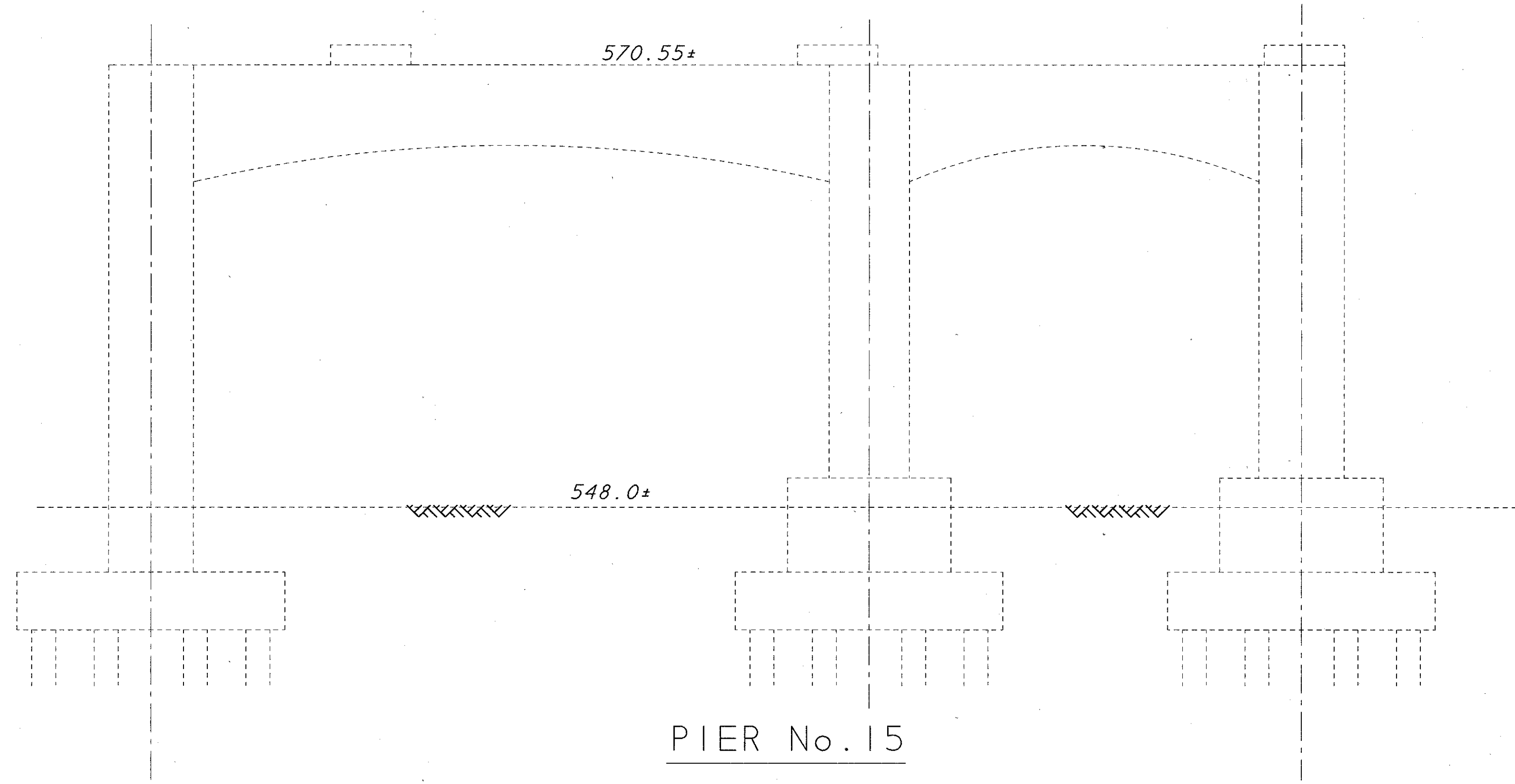
- Denotes Existing Structures
- Item Special -Patching Concrete Structures With Trowelable Mortar. (Total This Sh. 253 Sq. Ft.)

NOTES

- 1.-Area of patching shown on pier elevation represents the total patching area in pier.
- 2.-Representation of the existing structure is approximate.

CINCINNATI		Plum, Klausmeier & Gehrum 20/108 Consultants	
PATCHING CONCRETE STRUCTURES PIERS			
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)			
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19	
DESIGNED ED	DRAWN JDR	TRACED WDD	CHECKED IEH
		REVIEWED IEH	DATE April 96

PERS14 Scale 5:333



LEGEND

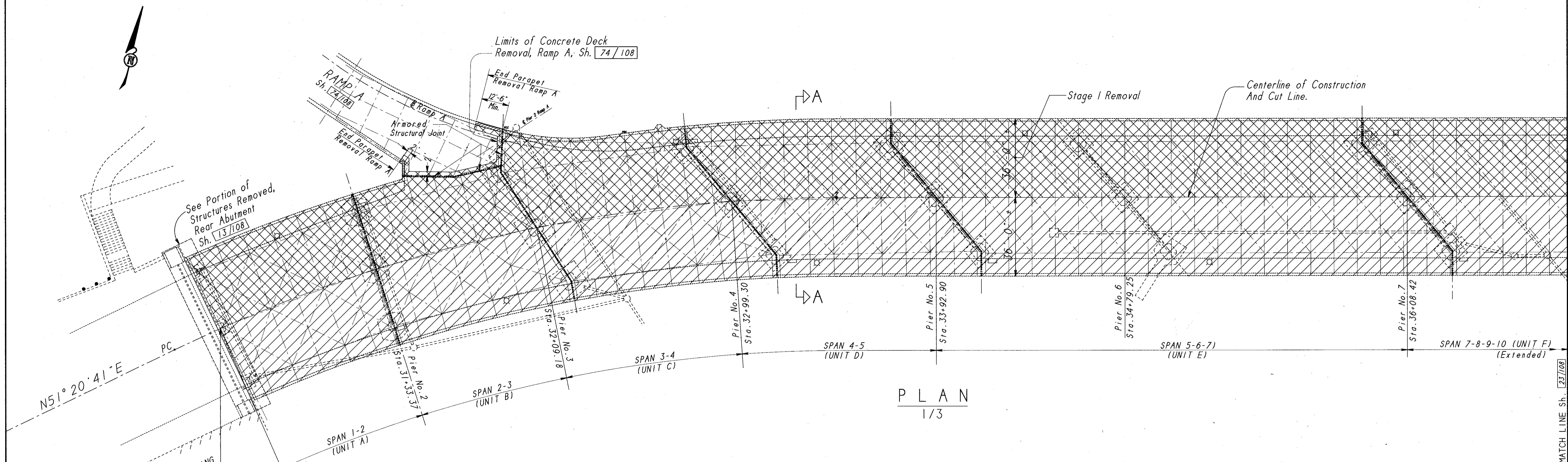
- Denotes Existing Structures
- Item Special -Patching Concrete Structures With Trowelable Mortar.
(Total This Sh. 38 Sq. Ft.)

NOTES

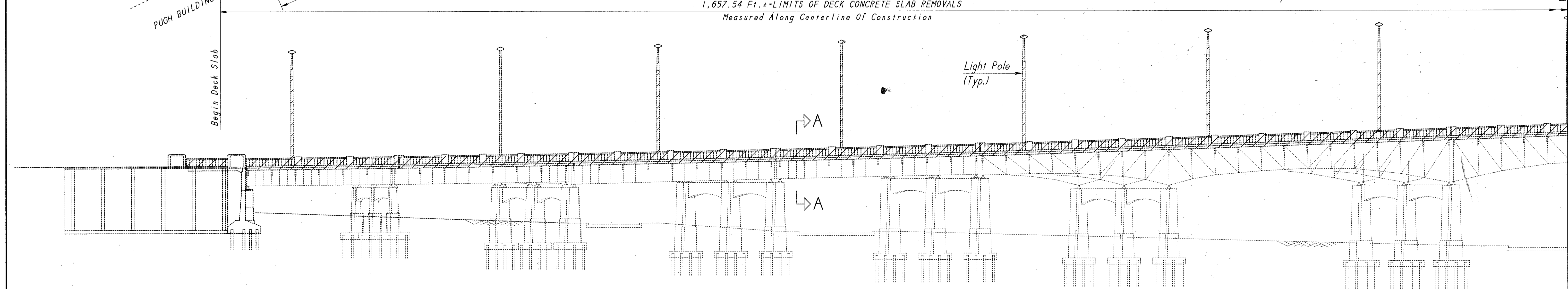
- 1.-Area of patching shown on pier elevation represents the total patching area in pier.
- 2.-Representation of the existing structure is approximate.

CINCINNATI		Plum, Klausmeter & Gehrum 21/108 Consultants	
PATCHING CONCRETE STRUCTURES PIERS			
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)			
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19	
DESIGNED ED	DRAWN JDR	CHECKED WDD	REVIEWED IEH April 96

PIERS/15 Scale 5/323



PLAN
1/3



ELEVATION
(LOOKING NORTH)
1/3

LEGEND

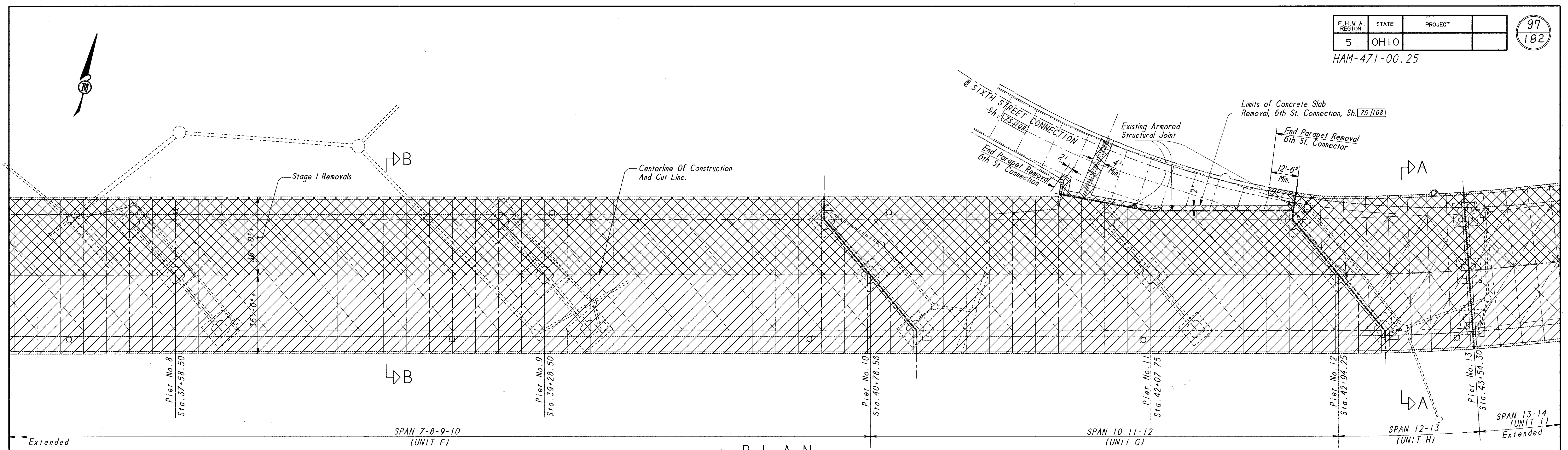
- Existing Concrete Structure To Be Removed
- Existing Concrete Structure To Be Removed As Stage I Construction

NOTES:

- 1 - Work this Sheet with Sh. 23/108 & Sh. 24/108
- 2 - See General Notes on Removals, Sh. 10/108
- 3 - Representation Of Existing Structure Is Approximate.

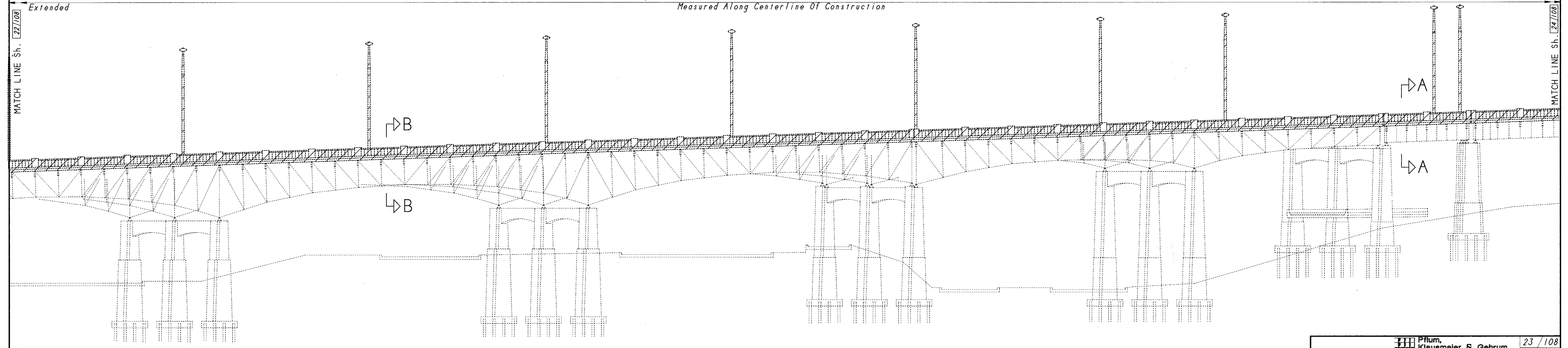
Plum. Klausmeier & Gehrung Consultants		22/108
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 1/3 UNITS A THRU F		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

DECREMENT Scale 21:3333



PLAN
2/3

1,657.54 Ft. ± - LIMITS OF DECK CONCRETE SLAB REMOVALS
Measured Along Centerline Of Construction



ELEVATION
(LOOKING NORTH)
2/3

LEGEND

- Existing Concrete Structure To Be Removed
- Existing Concrete Structure To Be Removed As Stage 1 Construction



NOTES:

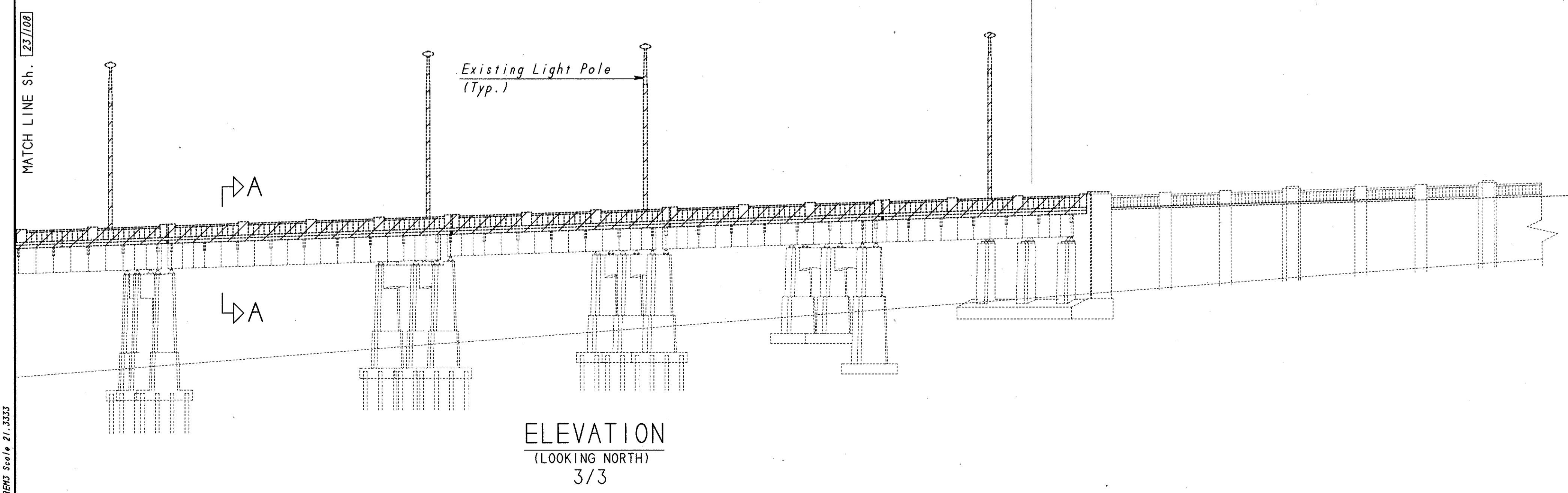
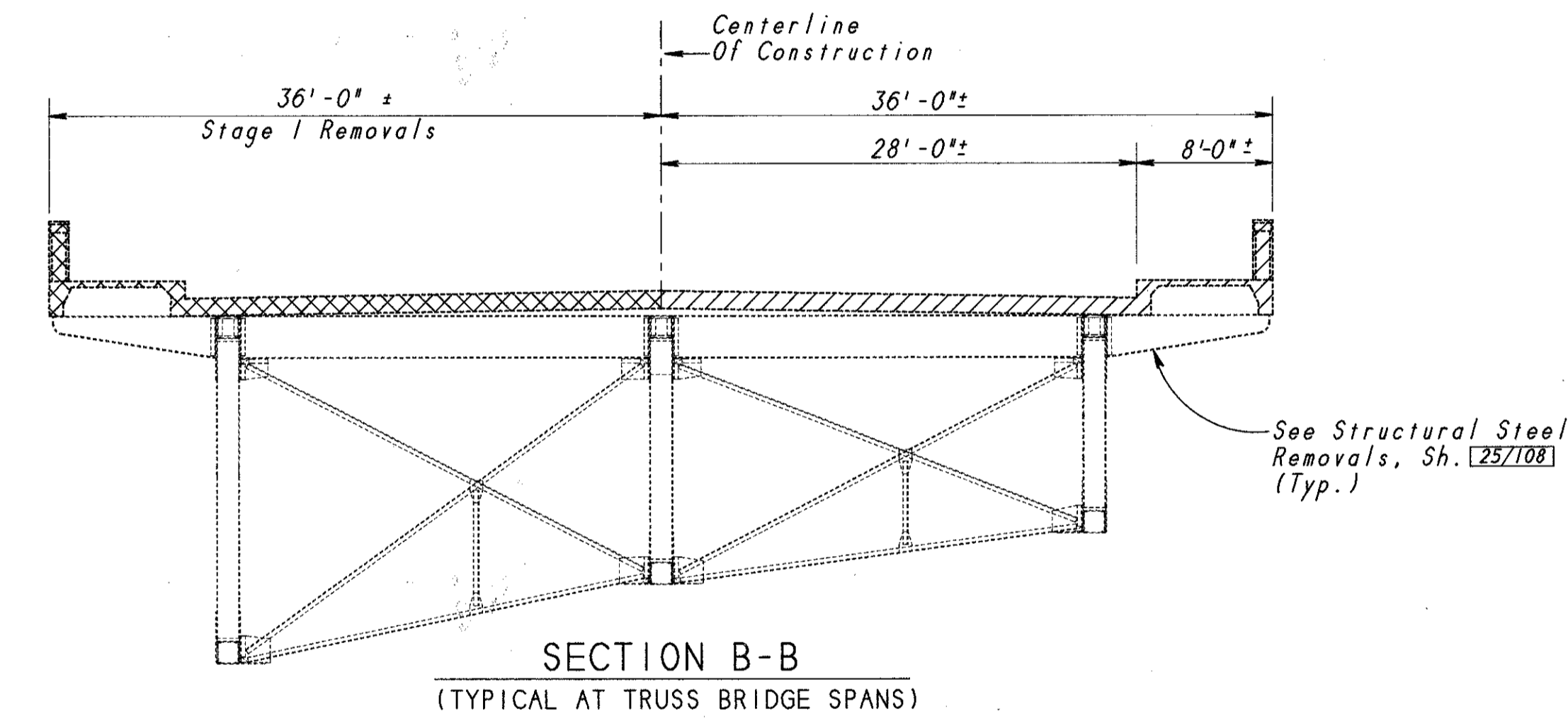
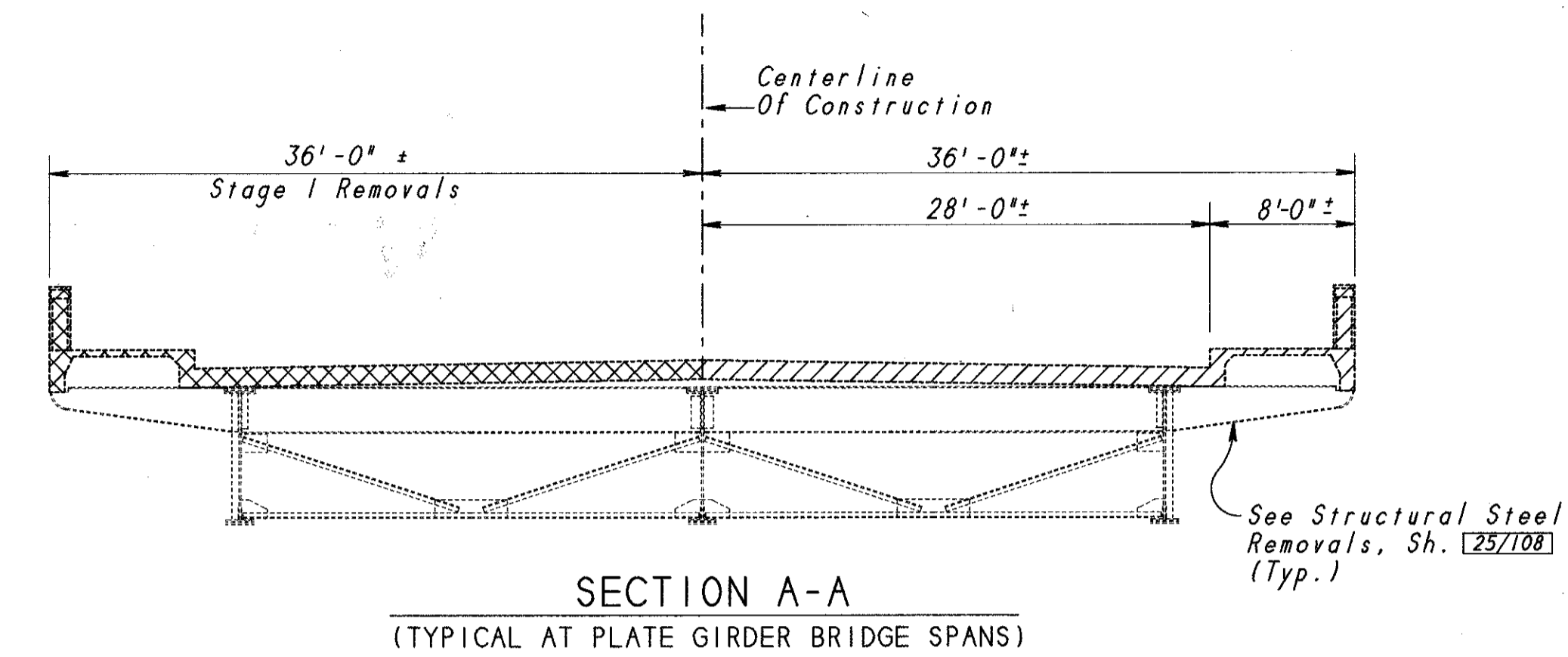
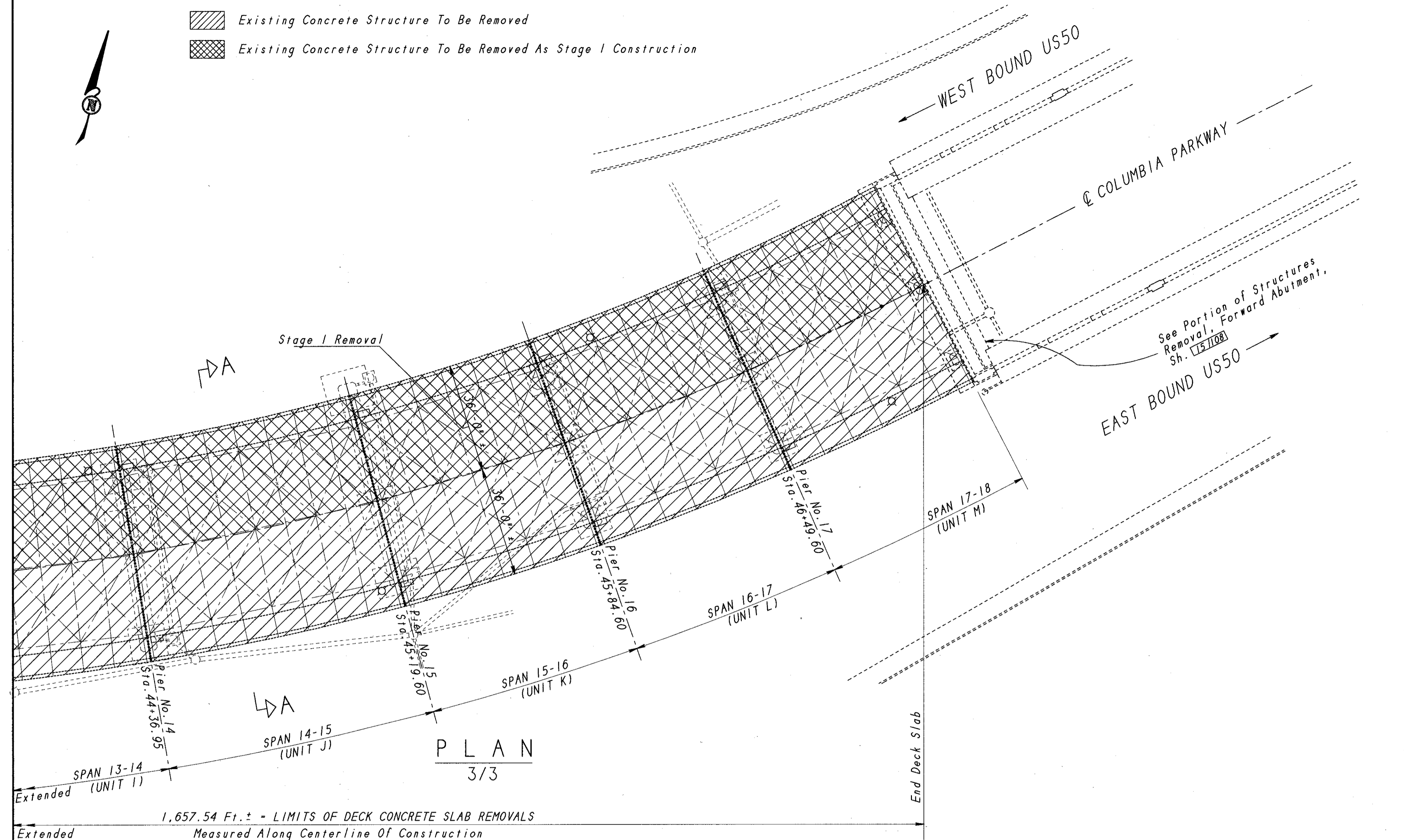
1. - Work this sheet with Sh. [22/1108] and Sh. [24/1108]
2. - See General Notes on Removals, Sh. [01/108]
3. - Representation Of Existing Structure Is Approximate.

Plum, Klausmeyer & Gehrmann Consultants		23 / 108 OHIO
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 2/3 UNITS F THRU I		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

DECK REMOVAL Scale 21-333

LEGEND

-  Existing Concrete Structure To Be Removed
-  Existing Concrete Structure To Be Removed As Stage 1 Construction

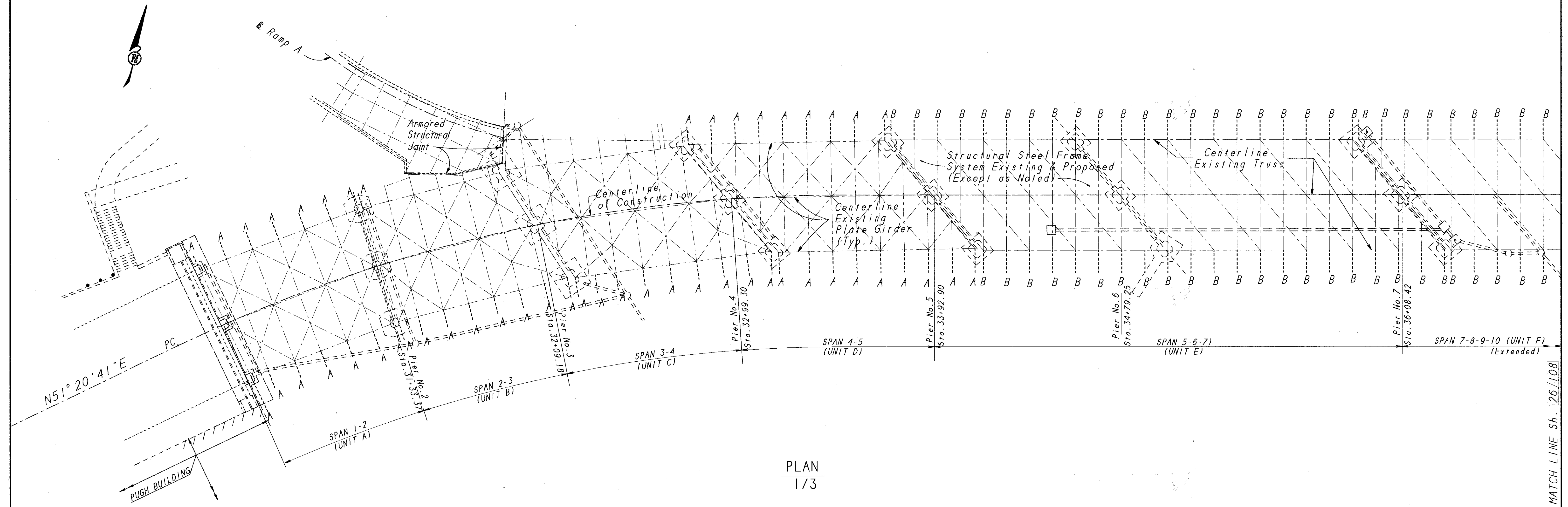


NOTES:

- 1 - Work this Sheet with Sh. [22/108] & Sh. [23/108]
- 2 - See General Notes on Removals, Sh. [10/108]

Plum, Klausmeyer & Gehrm Consultants		24/108
PORTIONS OF STRUCTURES REMOVED CONCRETE DECK SLAB 3/3 UNITS I THRU M		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

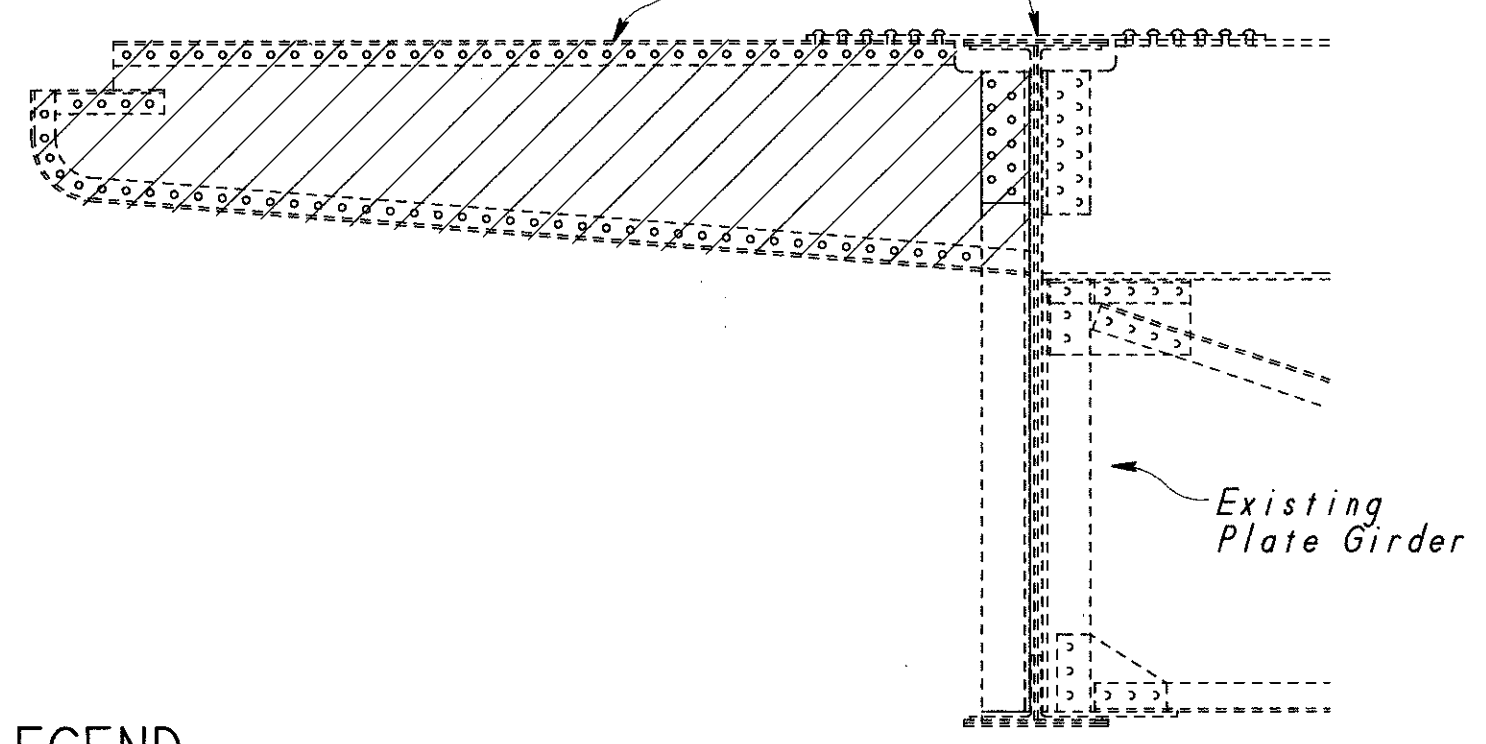
RECORDS Scale 21, 3333



PLAN
1/3

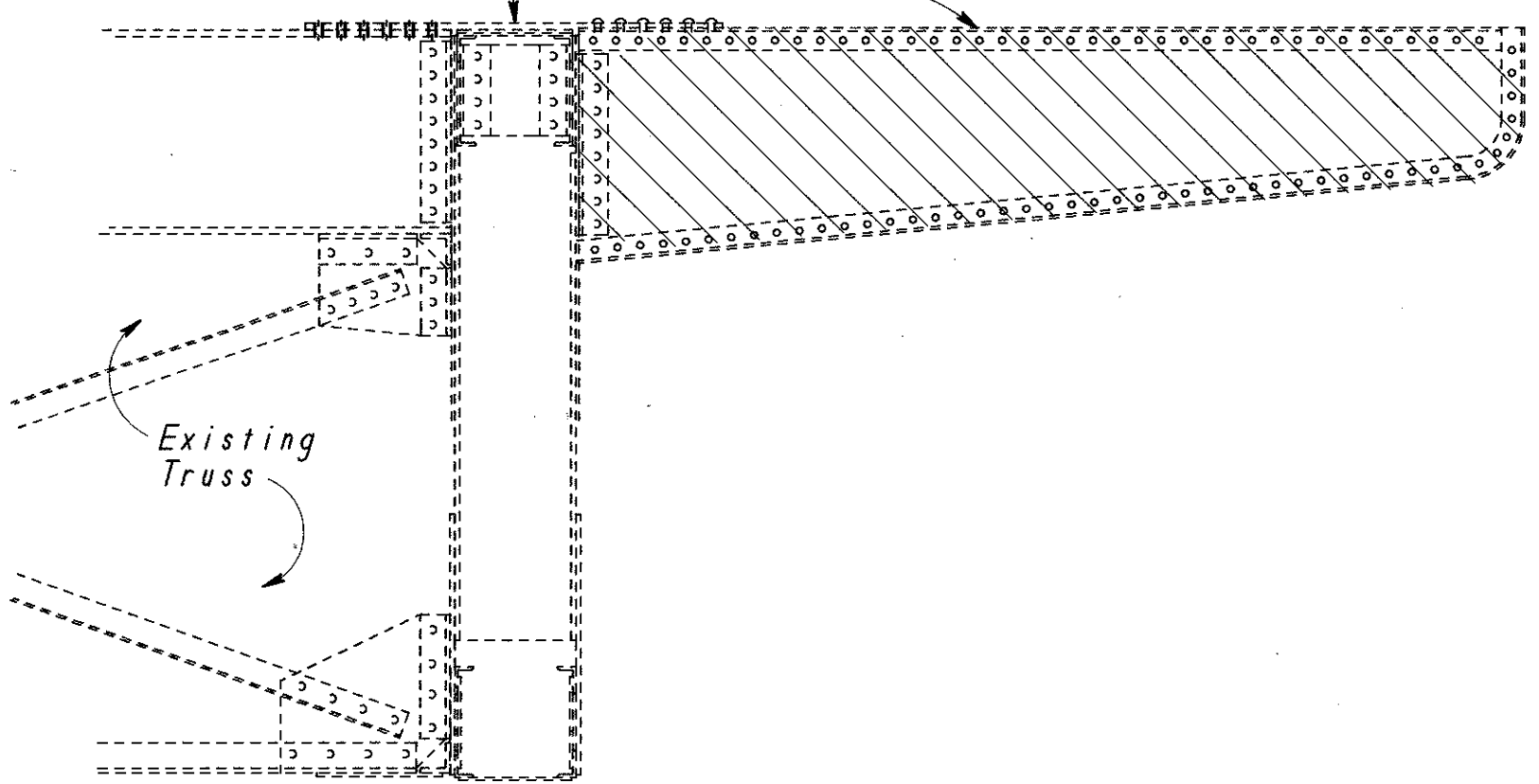
MATCH LINE Sh. 26/108

Remove Existing Tie Plate & Bracket (See Structural Steel Details Brackets, Sh. 45/108)



REMOVAL TYPE A

Remove Existing Tie Plate And Existing Bracket (See Structural Steel Details Sh. 46/108)



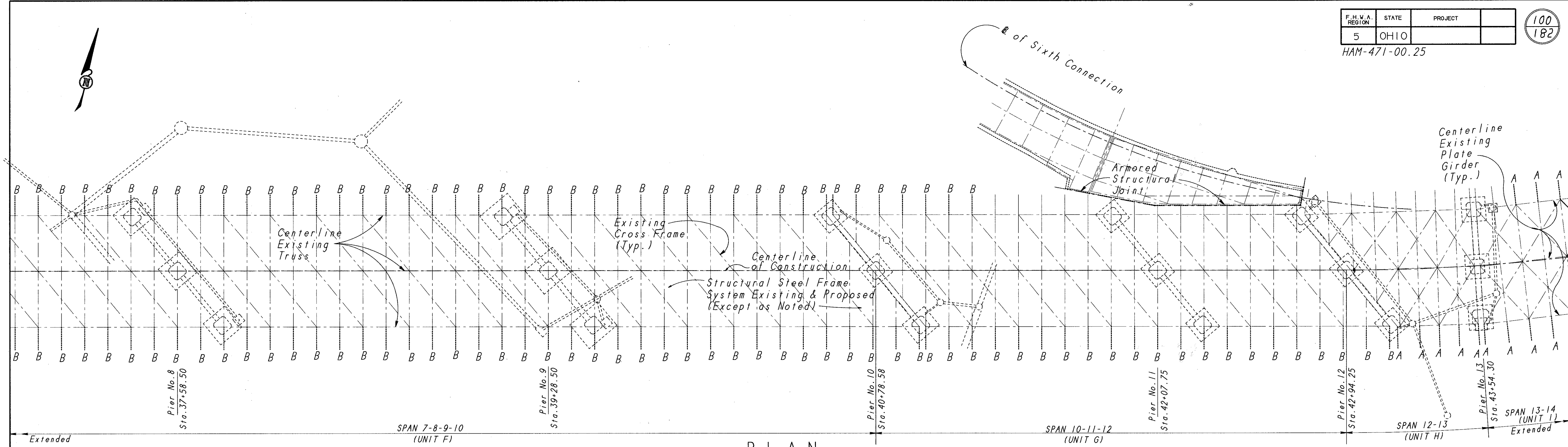
REMOVAL TYPE B

LEGEND

A Denotes Removal Type A
B Denotes Removal Type B

Plum, Klausmeier & Gehrum Consultants		25 / 108
STRUCTURAL STEEL DETAILS REMOVALS 1/3 UNITS A THRU F BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471) Sta. 30+55.40 Sta. 47+16.19		
HAMILTON COUNTY		
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

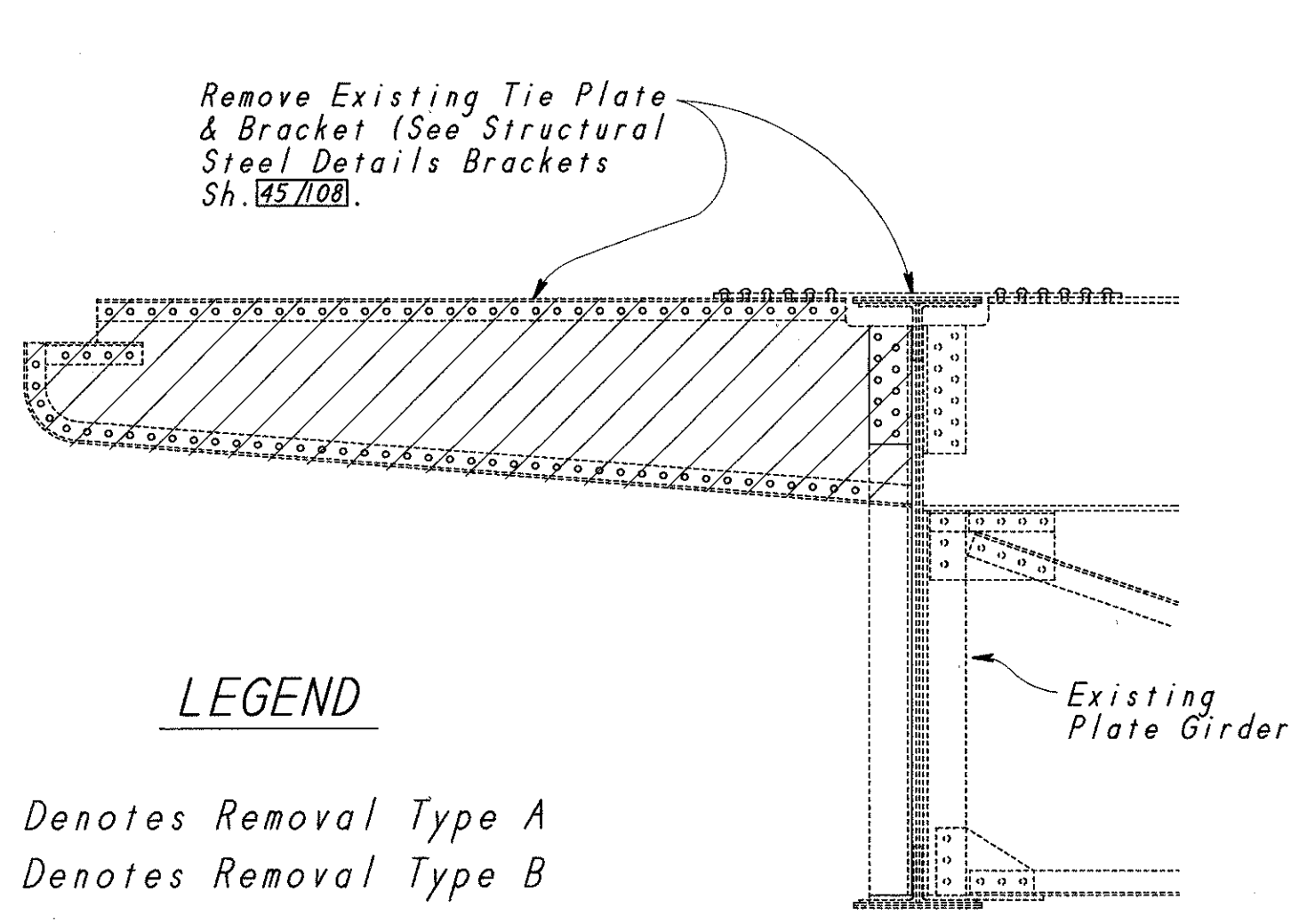
STRUCTURAL Scale 21/333



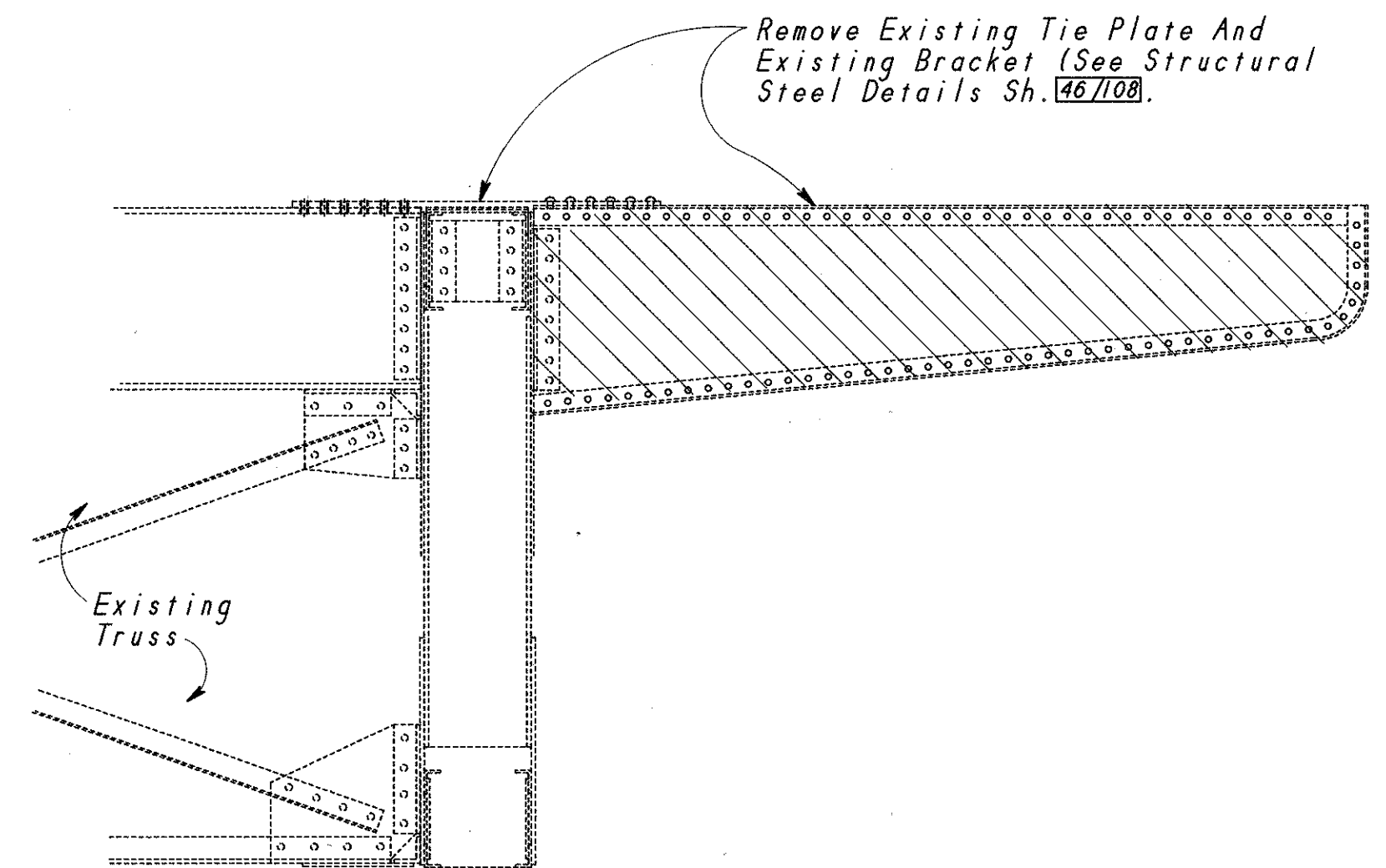
PLAN
2/3

MATCH LINE Sh. 25/108

MATCH LINE Sh. 27/108



REMOVAL TYPE A



REMOVAL TYPE B

LEGEND

A Denotes Removal Type A
B Denotes Removal Type B

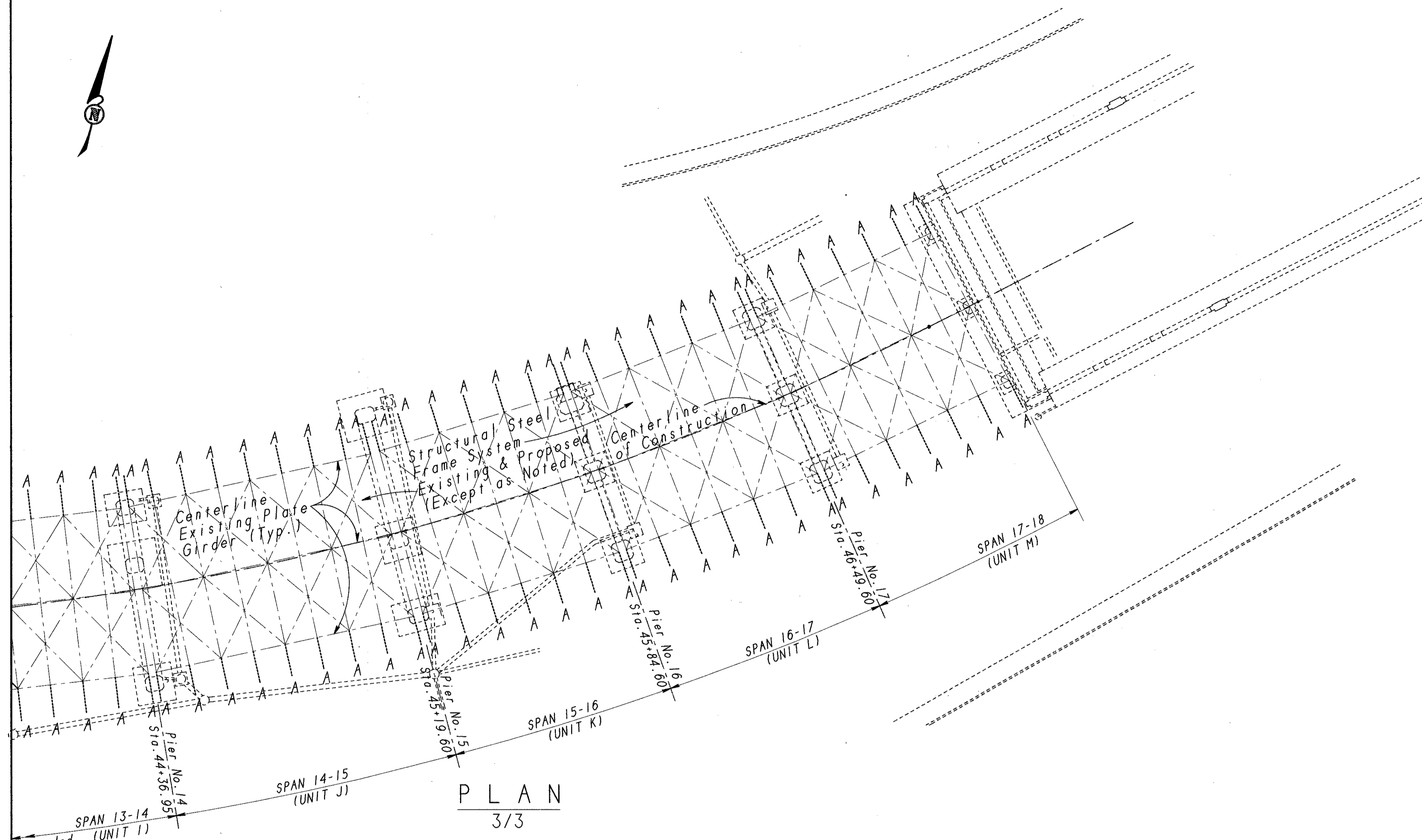
		26/108				
<p>STRUCTURAL STEEL DETAILS REMOVALS 2/3 UNITS F THRU I BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471) Sta. 30+55.40 Sta. 47+16.19 HAMILTON COUNTY</p>						
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED/JDR		WDD	IEH	April 96	

STRUCTURAL Scale 21/333

F. H. V. A. REGION	STATE	PROJECT	
5	OHIO		

101
182

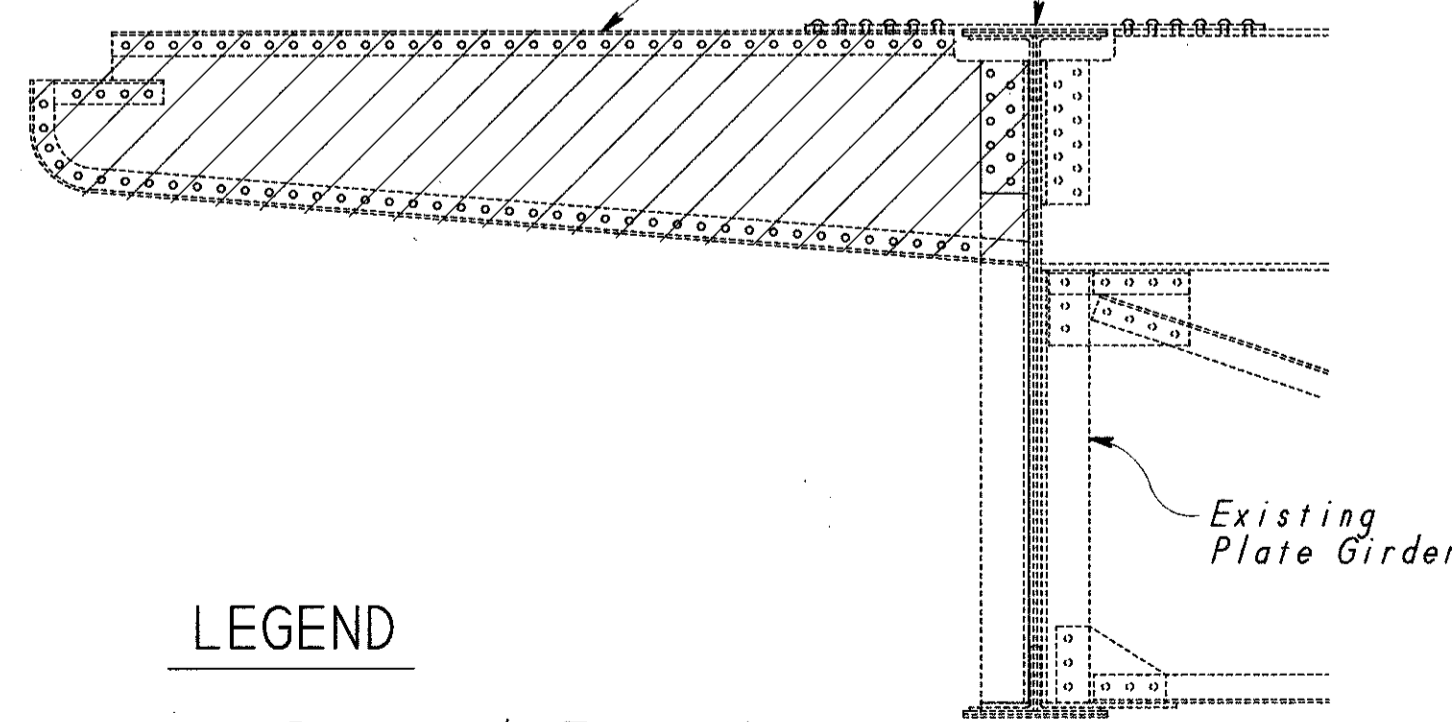
HAM-471-00.25



PLAN
3/3

MATCH LINE Sh. 26/108

Remove Existing Tie Plate & Bracket (See Structural Steel Details Brackets Sh. 45/108).



Existing Plate Girder

LEGEND

A Denotes Removal Type A

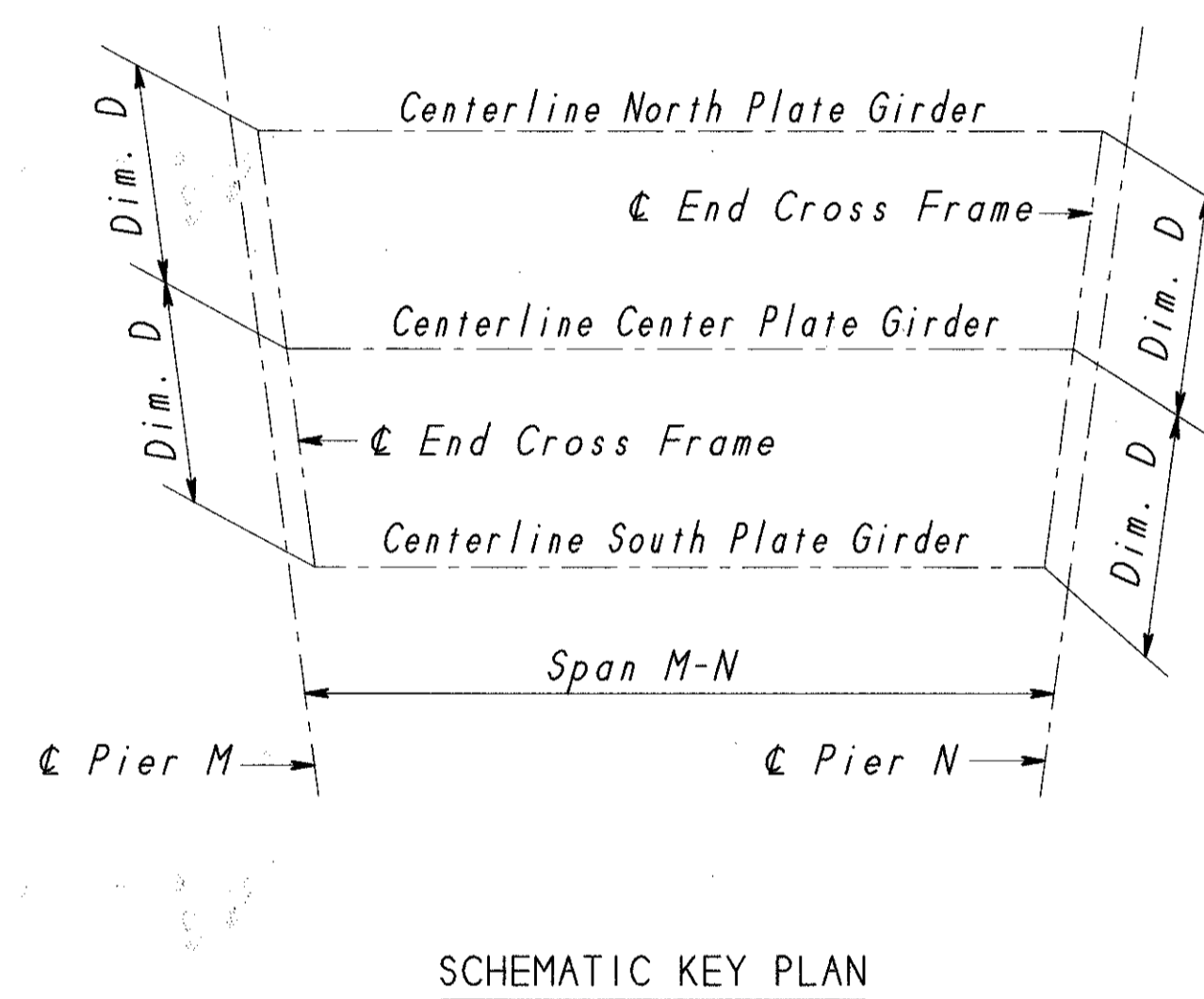
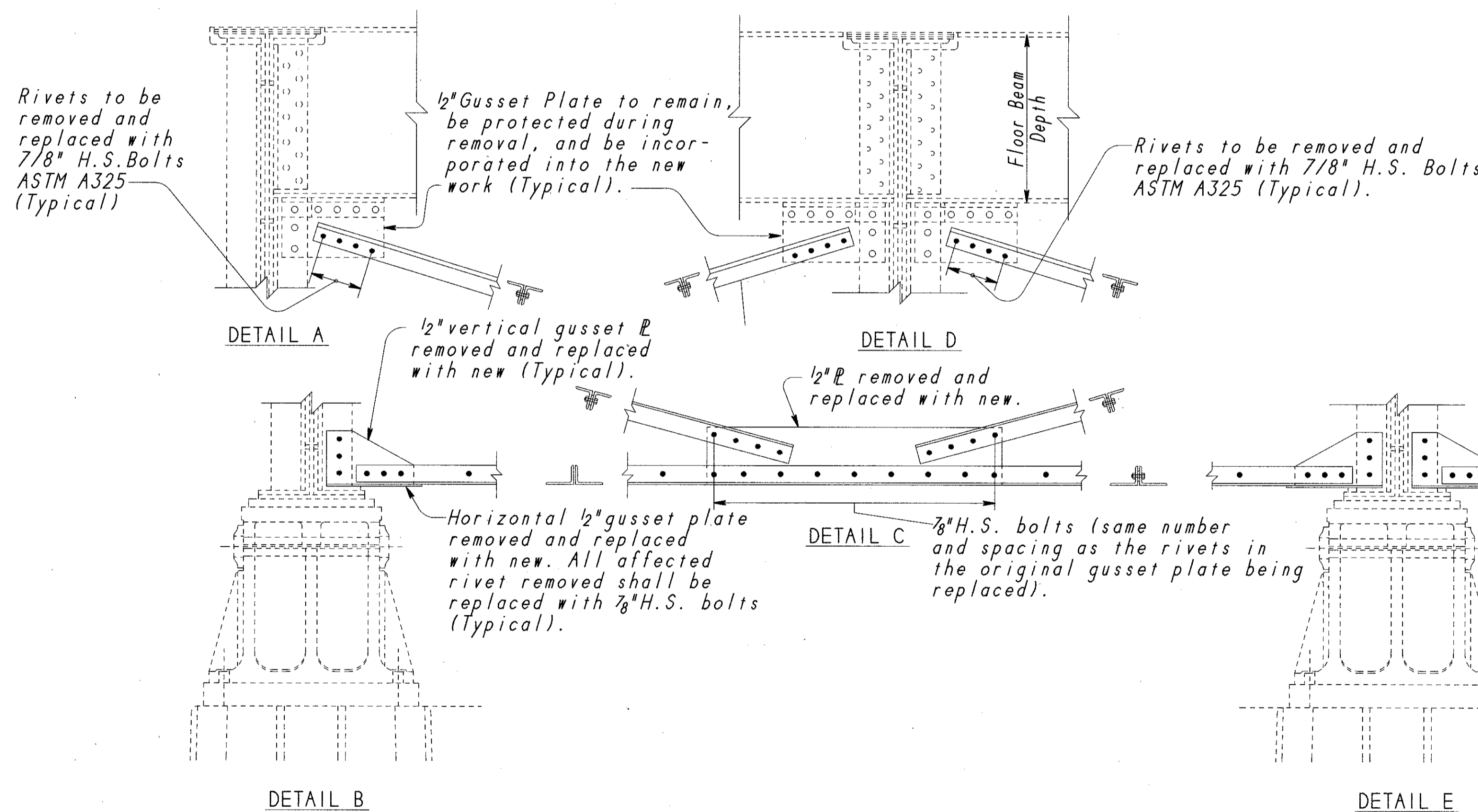
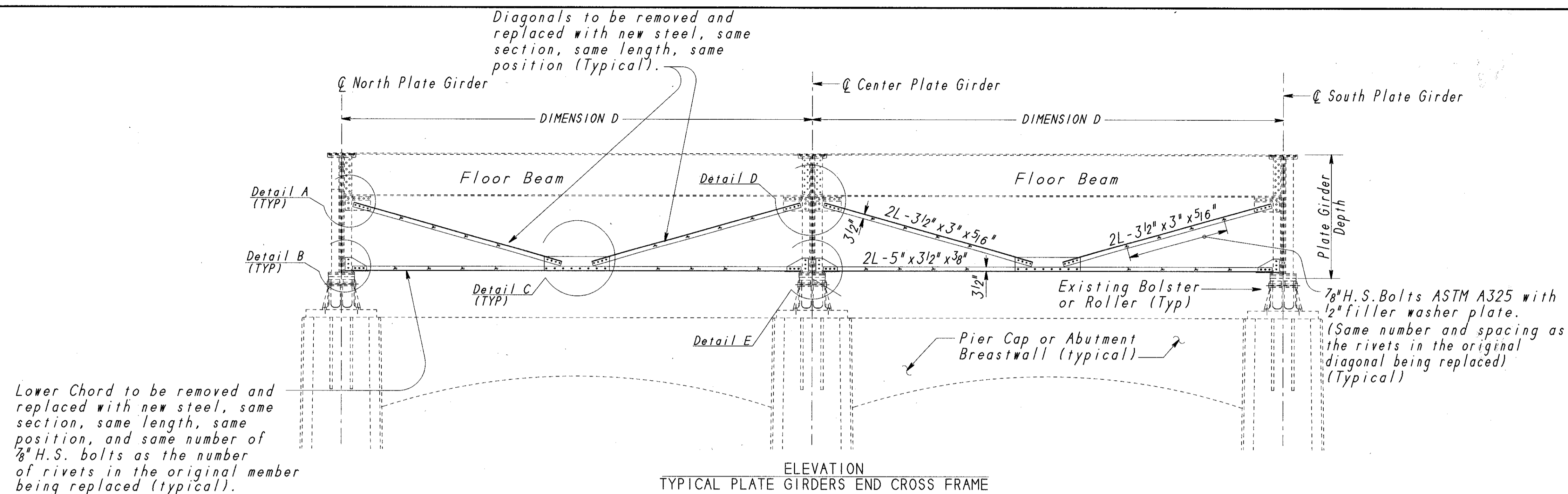
REMOVAL TYPE A

CINCINNATI Pfum, Klausmeyer & Gehrum 27/108
Consultants OHIO

STRUCTURAL STEEL DETAILS
REMOVALS 3/3
UNITS I THRU M
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)
Sta. 30+55.40
Sta. 47+16.19
HAMILTON COUNTY

DESIGNED ED	DRAWN ED/JDR	TRACED	CHECKED WDD	REVIEWED IEH	DATE April 96	REVISED
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STRUCT35A Scale 21-333



LEGEND

Existing Structure To Remain.

New Work.

NOTES:

1. - See Note Item 513-Structural Steel Replacement Of Deteriorated End Cross Frames In General Notes.

2. - See GENERAL NOTES, Sh. 7 / 108 Thru Sh. 11 / 108

SPAN	DIMENSION D			
	Pier No.	DIM. D	Pier No.	DIM. D
1-2	Rear Abut.	27'-5 1/4" ±	No. 2	27'-4 1/2" ±
2-3	No. 2	27'-4 1/2" ±	No. 3	28'-9 7/8" ±
3-4	No. 3	29'-7" ±	No. 4	31'-7 1/4" ±
4-5	No. 4	32'-4 3/4" ±	No. 5	32'-4 3/4" ±
12-13	No. 12	33'-2 9/16" ±	No. 13	26'-2 3/16" ±
13-14	No. 13	26'-2 3/8" ±	No. 14	26'-2 3/8" ±
14-15	No. 14	26'-2 3/8" ±	No. 15	26'-2 3/8" ±
15-16	No. 15	26'-2 1/4" ±	No. 16	26'-2 1/4" ±
16-17	No. 16	26'-2 1/4" ±	No. 17	26'-2 1/4" ±
17-18	No. 17	26'-2 1/4" ±	Frwd. Abut.	26'-2 1/4" ±

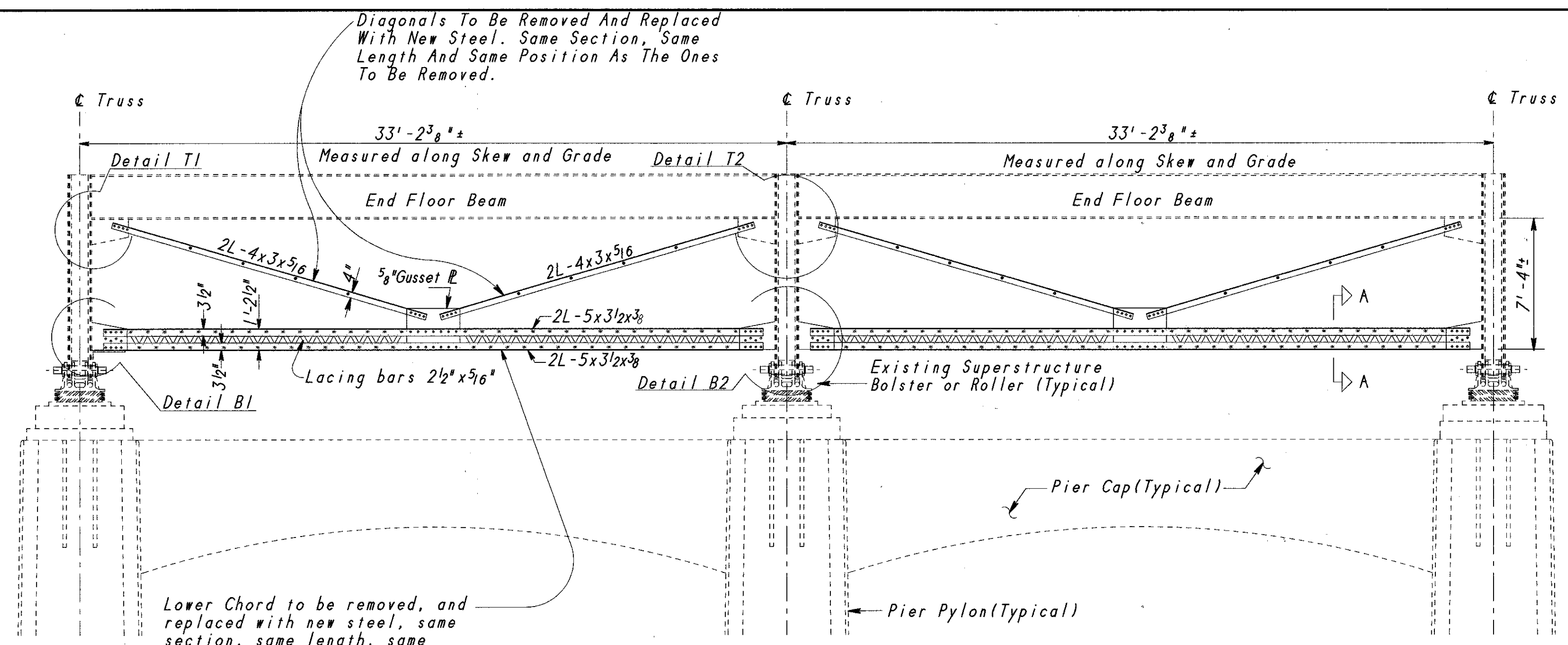
Plum, Klausmeyer & Gehrmann
Consultants

28 / 108

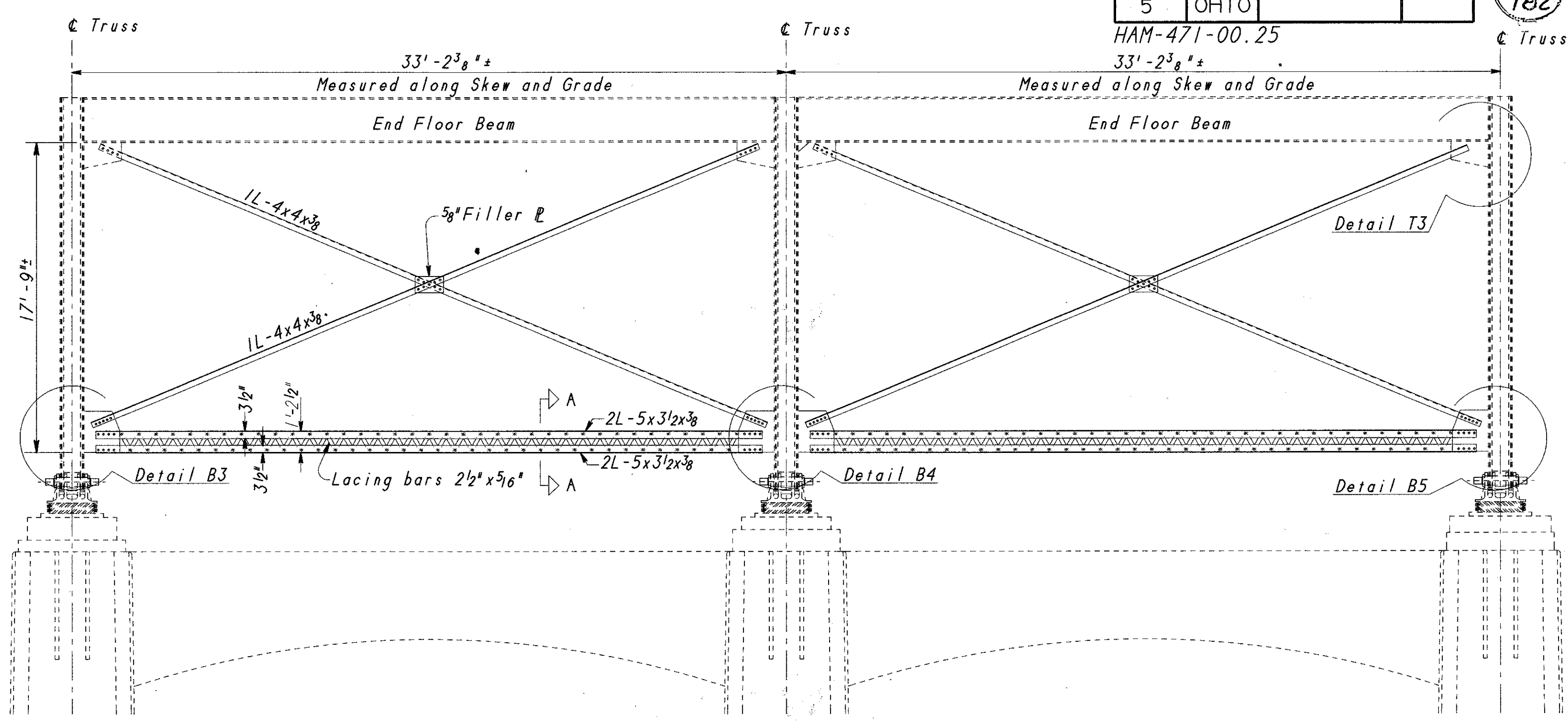
STRUCTURAL STEEL
REPLACEMENT OF DETERIORATED
END CROSS FRAMES
(PLATE GIRDER SPANS)
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
DESIGNED ED
DRAWN ED/PLF
TRACED
CHECKED WDD
REVIEWED IEH
DATE April 96
REVISION

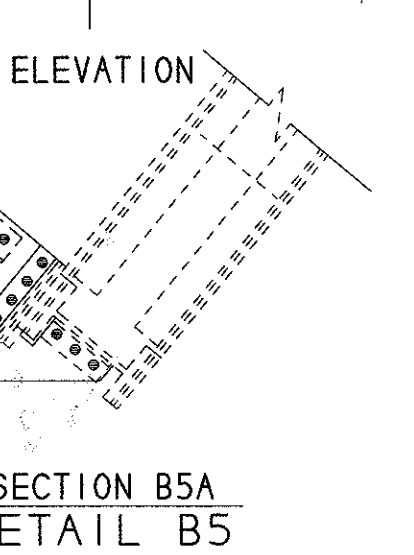
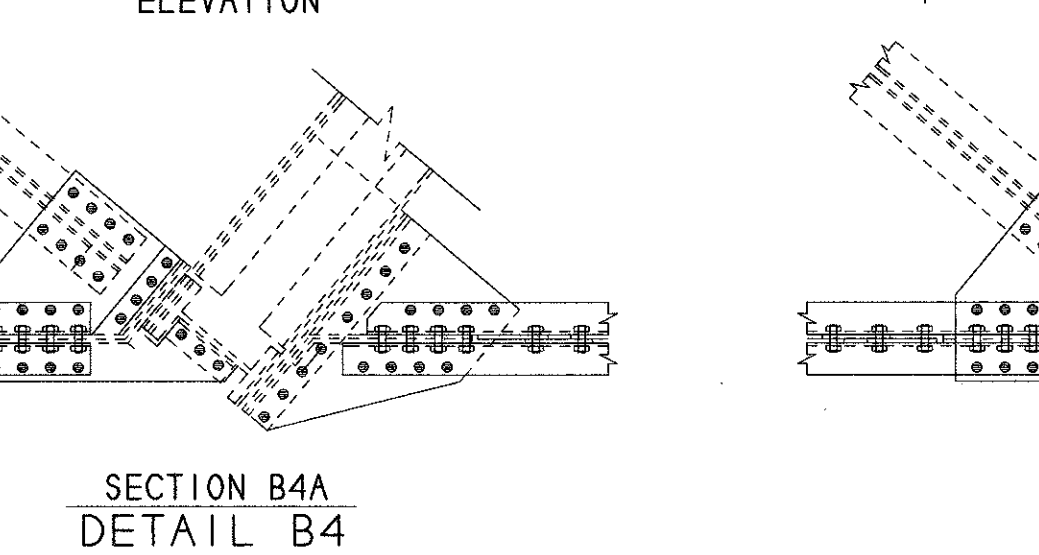
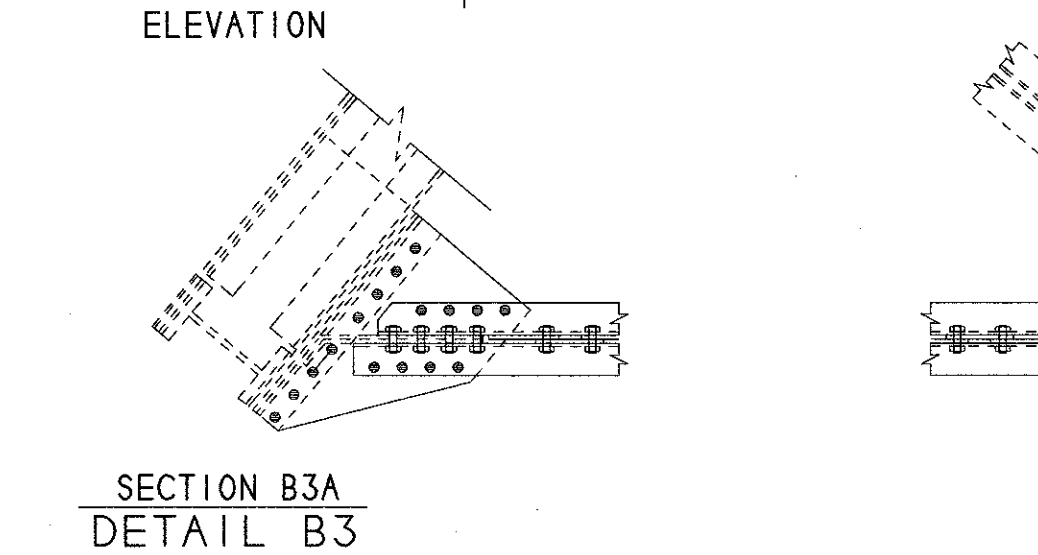
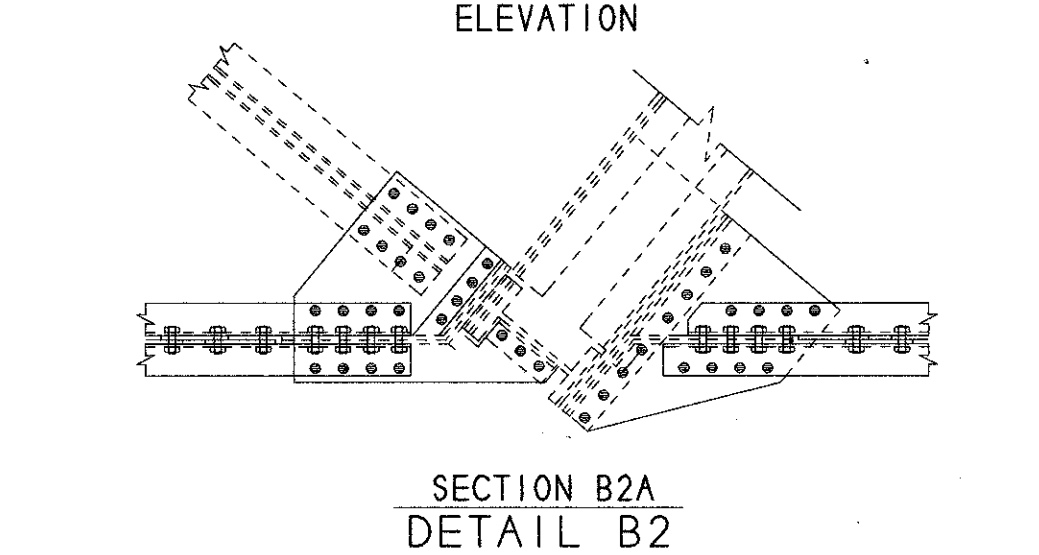
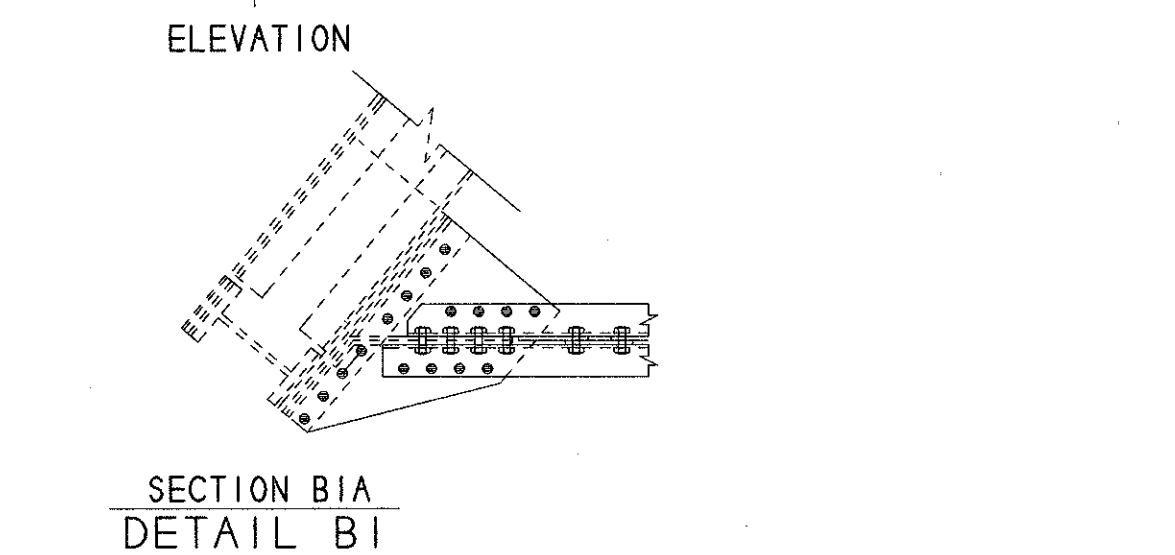
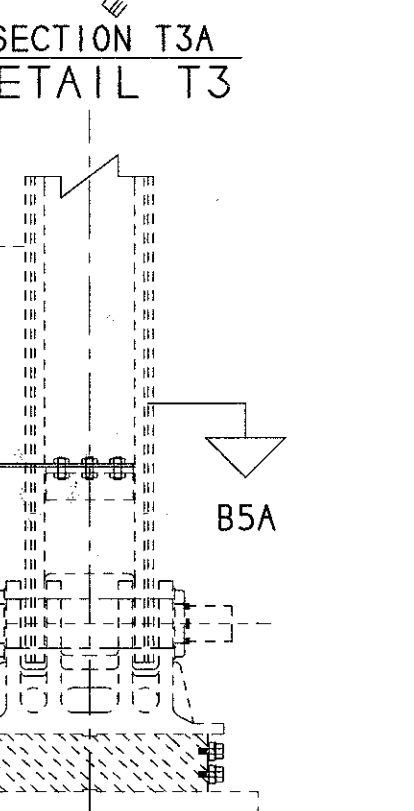
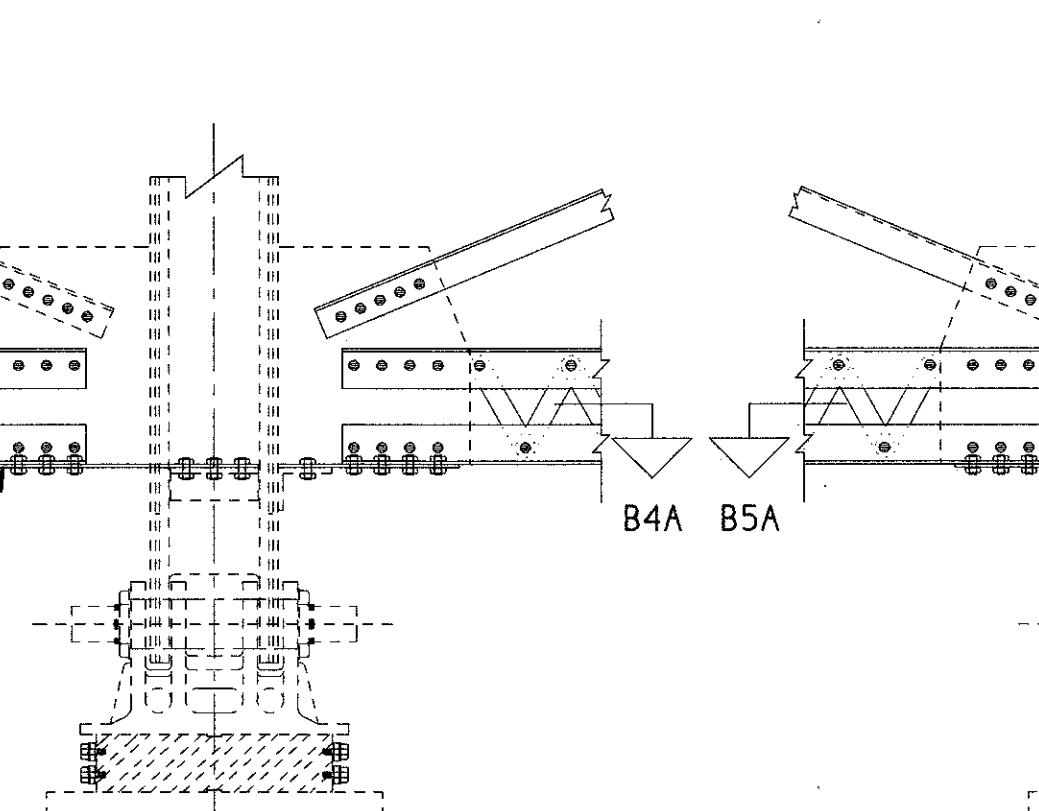
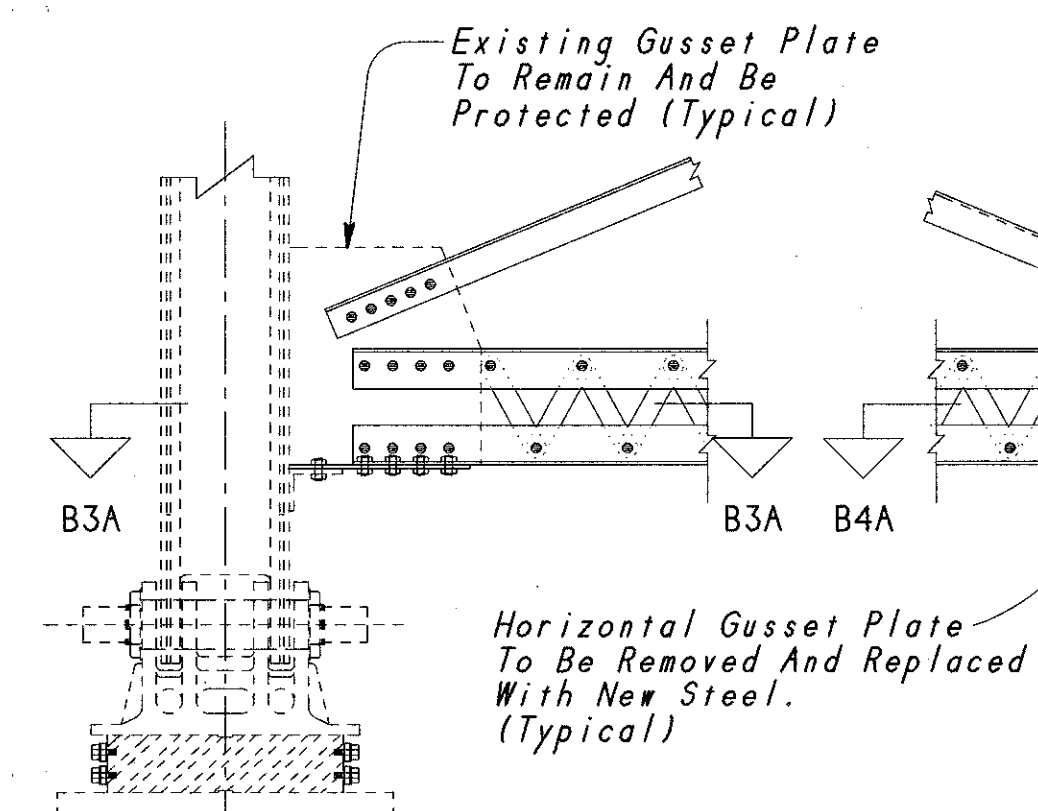
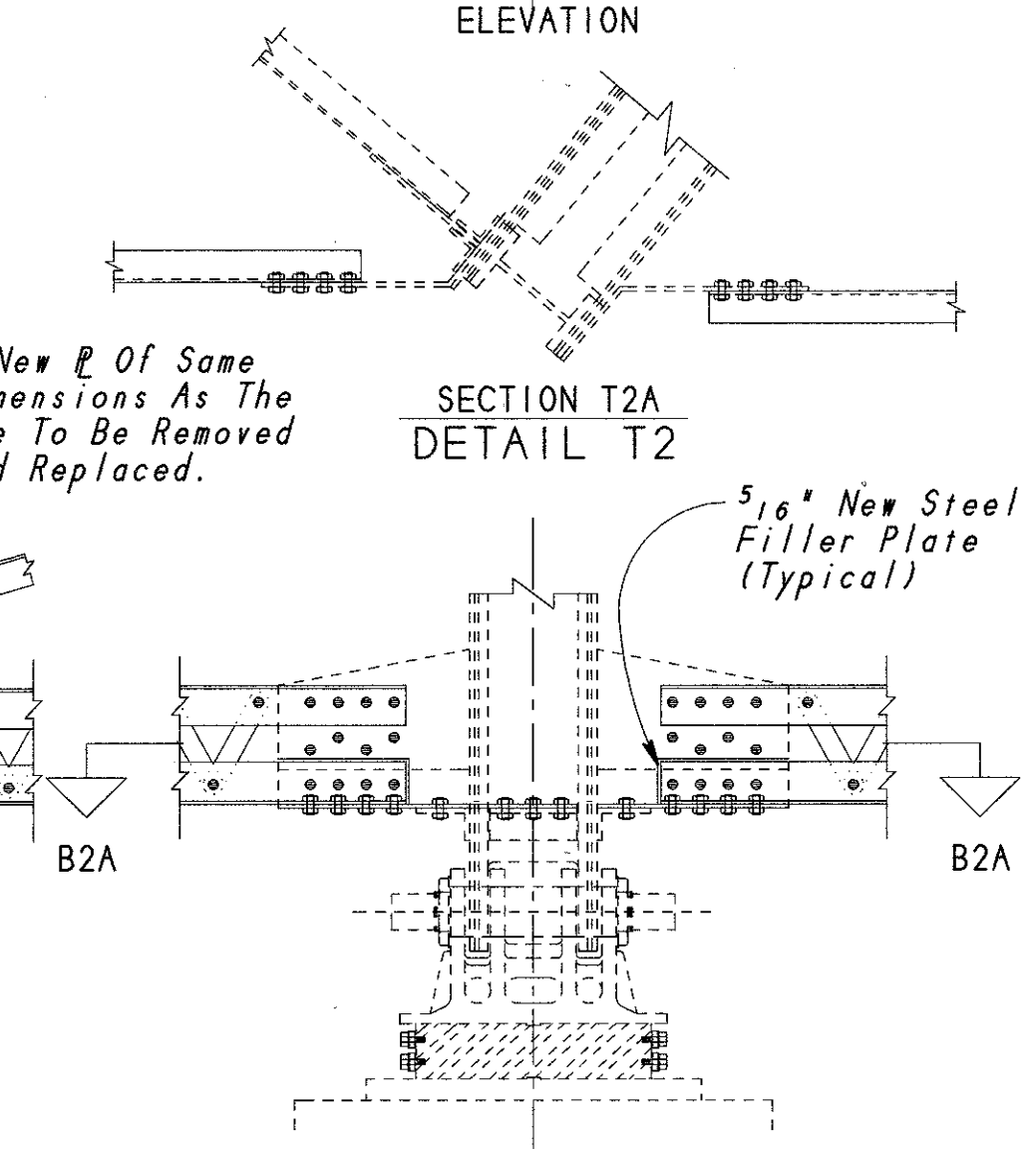
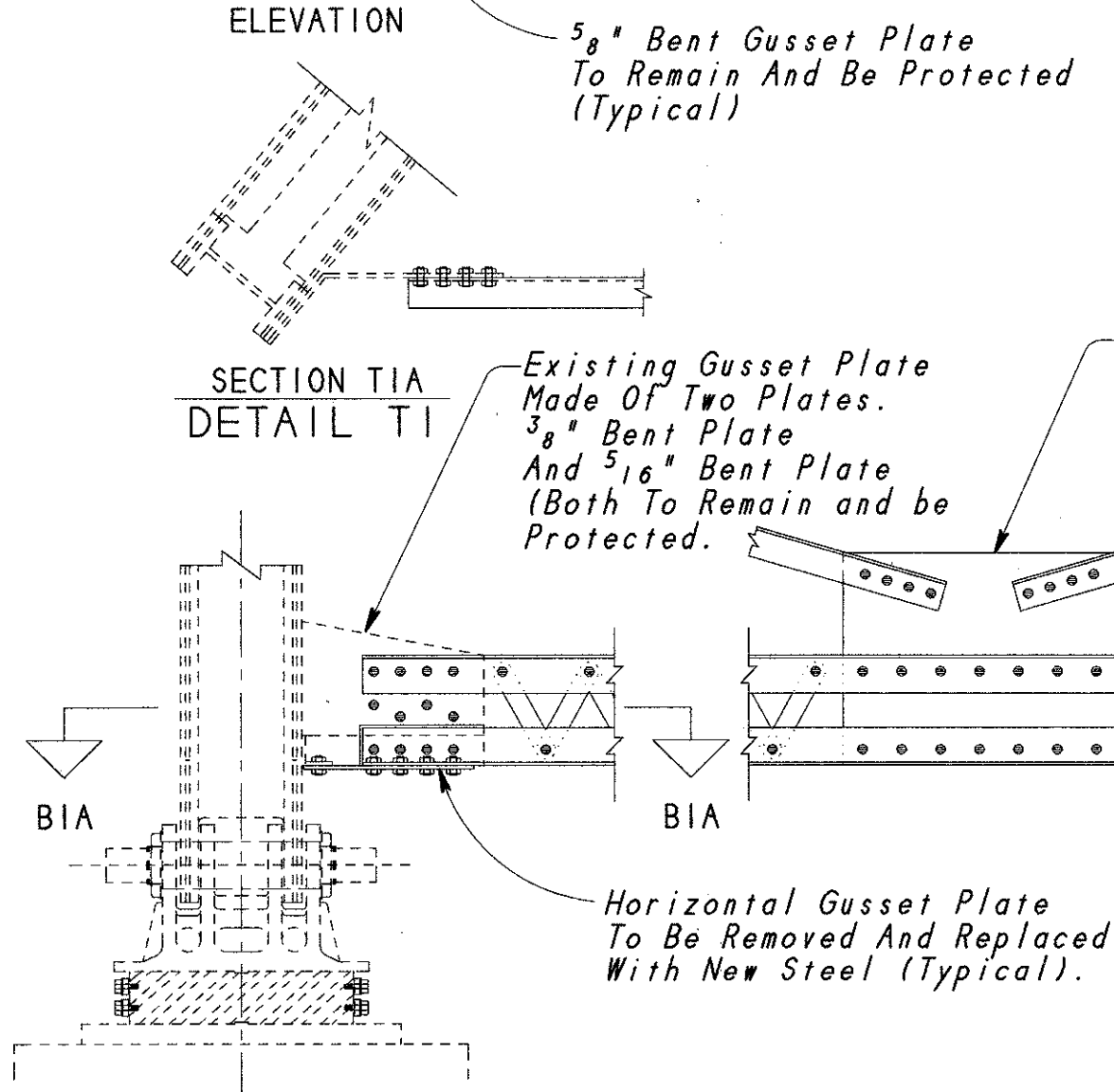
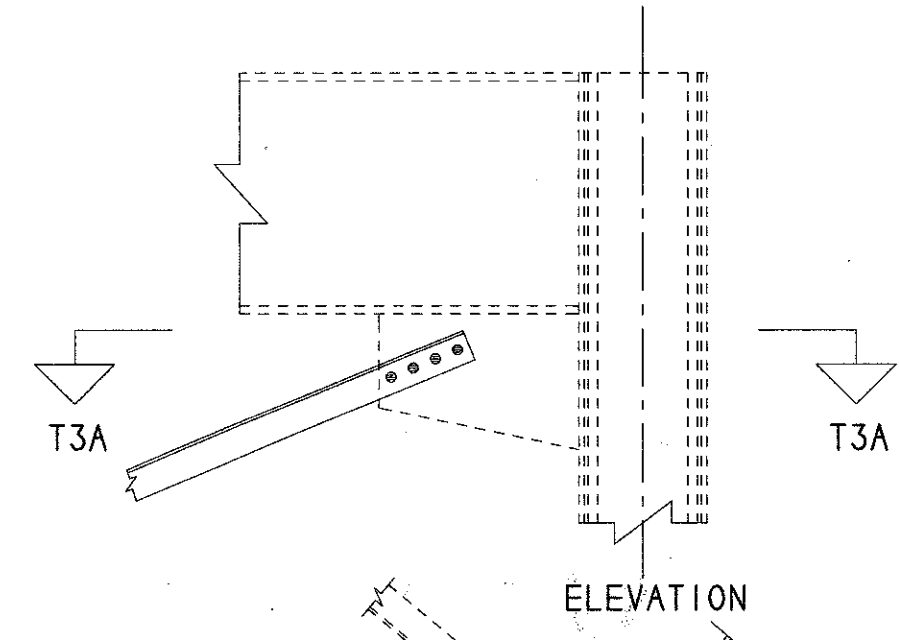
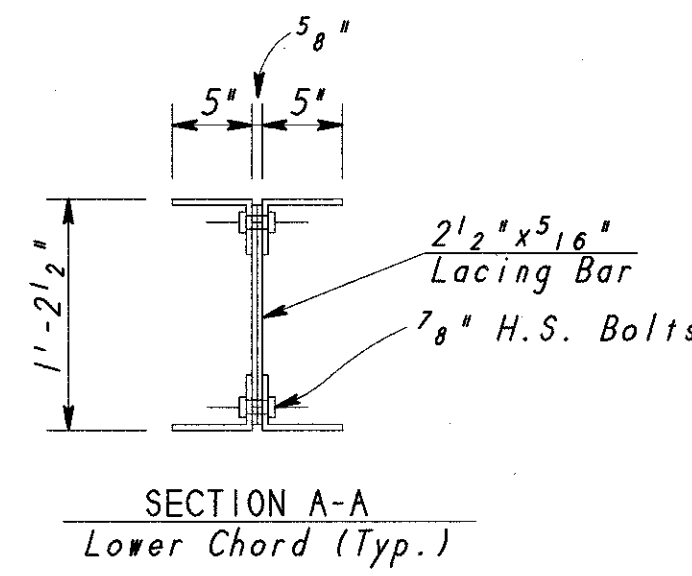
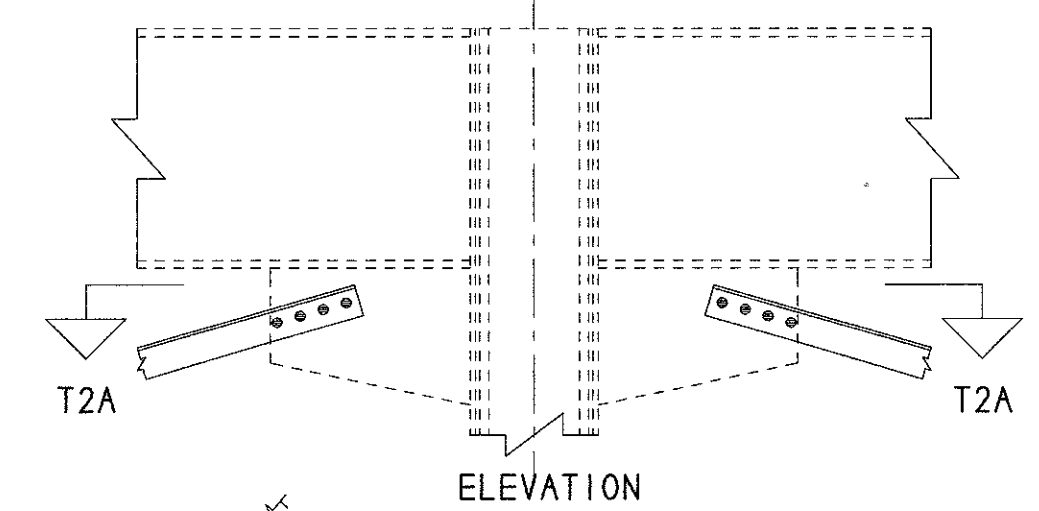
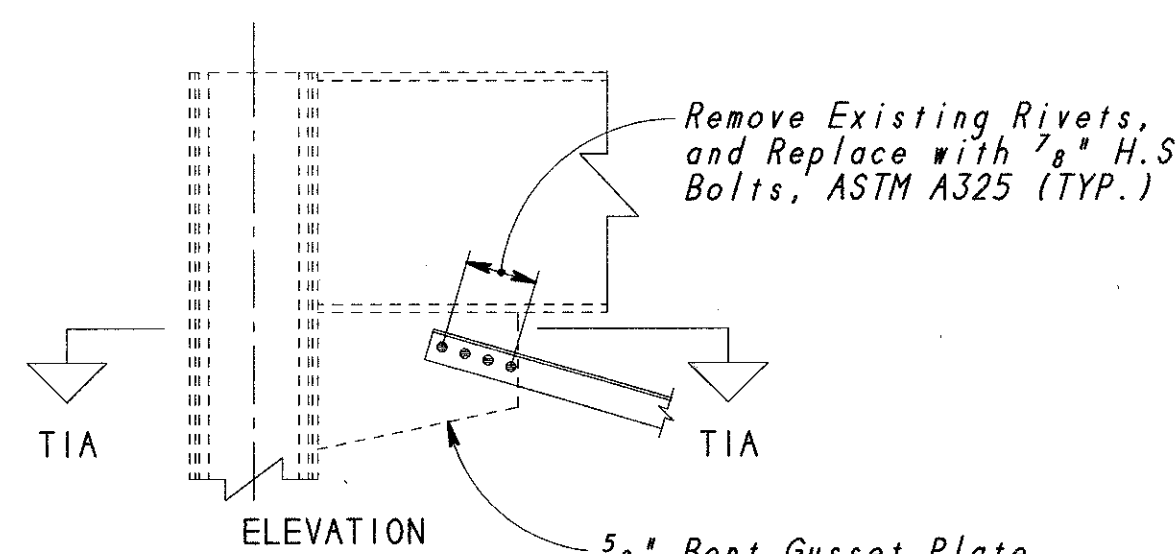
Sta. 30+55.40
Sta. 47+16.19



TRUSS SPAN END CROSS FRAMES AT PIER No. 5 AND No. 12
NUMBER OF FULL END-CROSS FRAMES REQUIRED-2



TRUSS SPAN END CROSS FRAMES AT PIER No. 7 AND No. 10
NUMBER OF FULL END-CROSS FRAMES REQUIRED-4



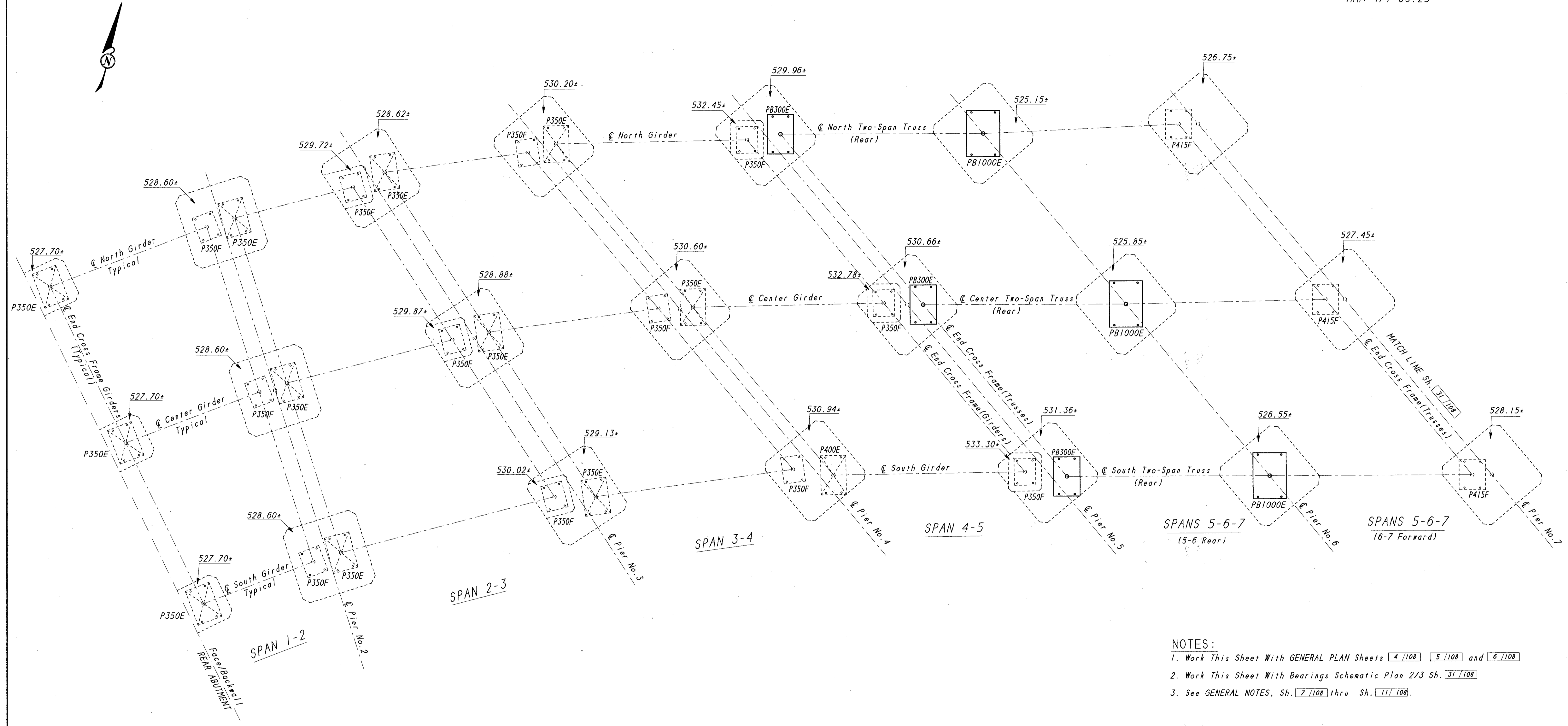
- NOTES:
- See Note Item 513-Structural Steel-Replacement Of Deteriorated End Cross Frames, As Per Plan, In General Notes, Sh. 10/108.
 - For GENERAL NOTES, See Sh. 7/108 Thru Sh. 11/108.

LEGEND

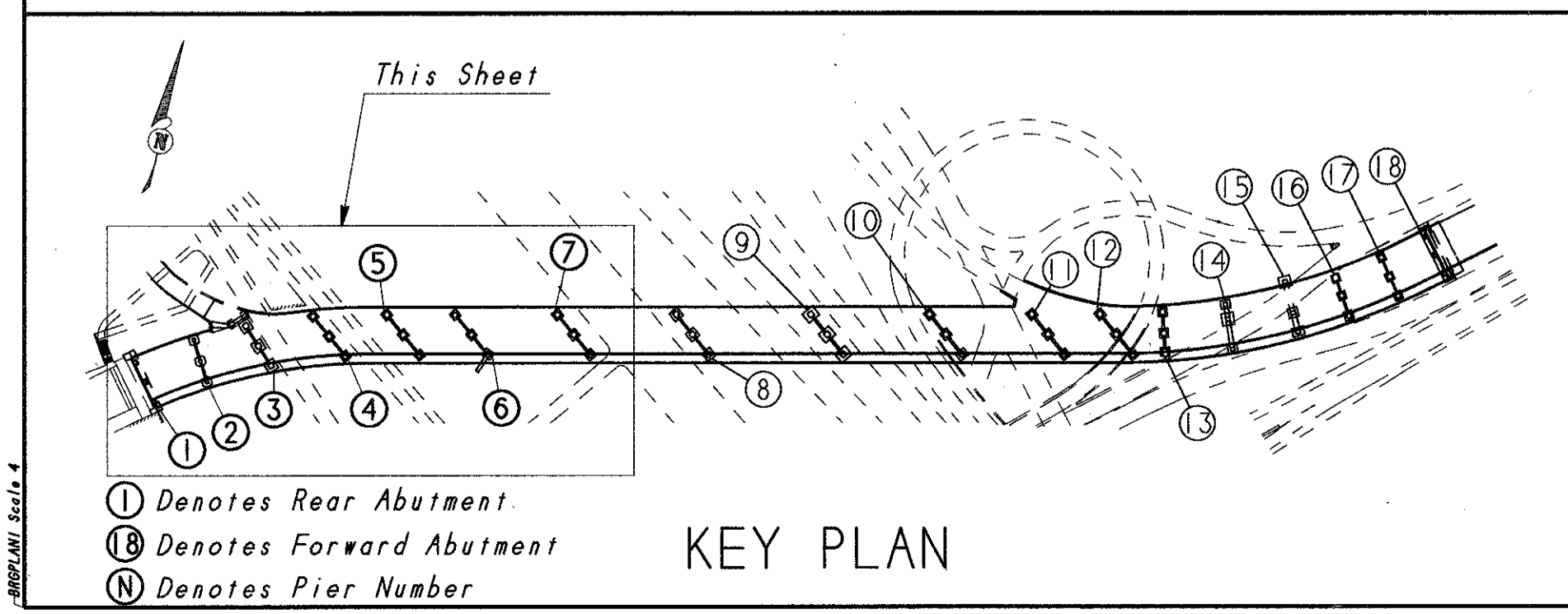
	Existing structure to remain.
	New work.

Pflum, Klausmeyer & Gehrm Consultants		29/108
STRUCTURAL STEEL REPLACEMENT OF DETERIORATED END CROSS FRAMES (TRUSS SPANS) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

CROSS Scale 1



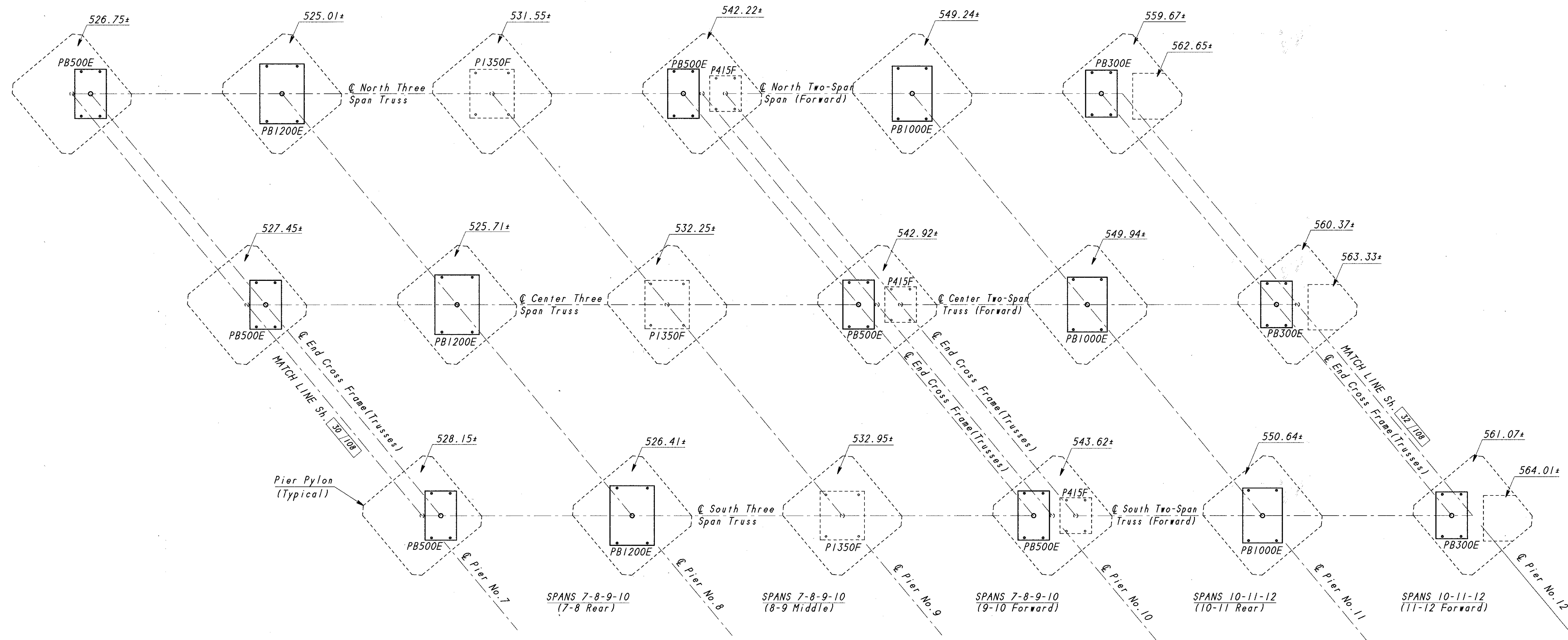
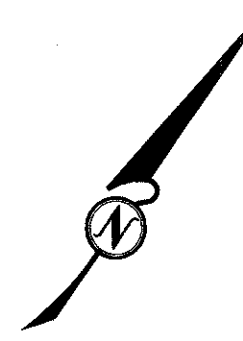
NOTES:
 1. Work This Sheet With GENERAL PLAN Sheets 4/108, 5/108 and 6/108
 2. Work This Sheet With Bearings Schematic Plan 2/3 Sh. 31/108
 3. See GENERAL NOTES, Sh. 7/108 thru Sh. 11/108.



LEGEND

- Denotes Existing Bearing.
- Denotes elastomeric bearing retrofit
- Denotes New Pot Bearing Device.
- 530.94± Denotes Masonry Elevation at top of pier pylon.
- PB Denotes POT BEARING
- P---F Denotes Existing FIXED Bearing
- P---E Denotes Existing EXPANSION Bearing

Pflum, Klausmeier & Gehrum Consultants		30/108
BEARING DEVICES SCHEMATIC PLAN 1/3 REAR ABUTMENT THRU PIER NO. 7		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

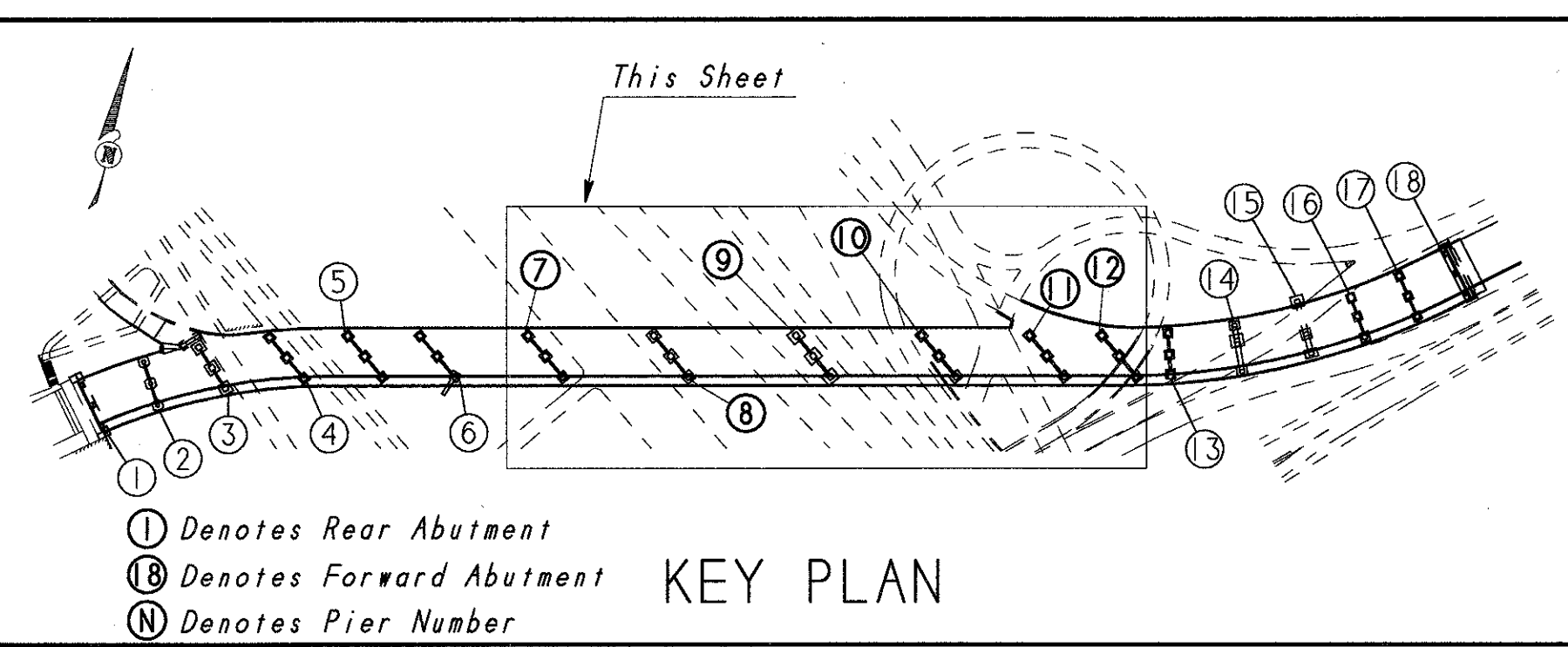


LEGEND

- Denotes Existing Bearing.
- Denotes New Pot Bearing Device.
- 530.94± Denotes Masonry Elevation at top of pier pylon.
- PB Denotes POT BEARING.
- P---F Denotes Existing FIXED Bearing
- P---E Denotes Existing EXPANSION Bearing

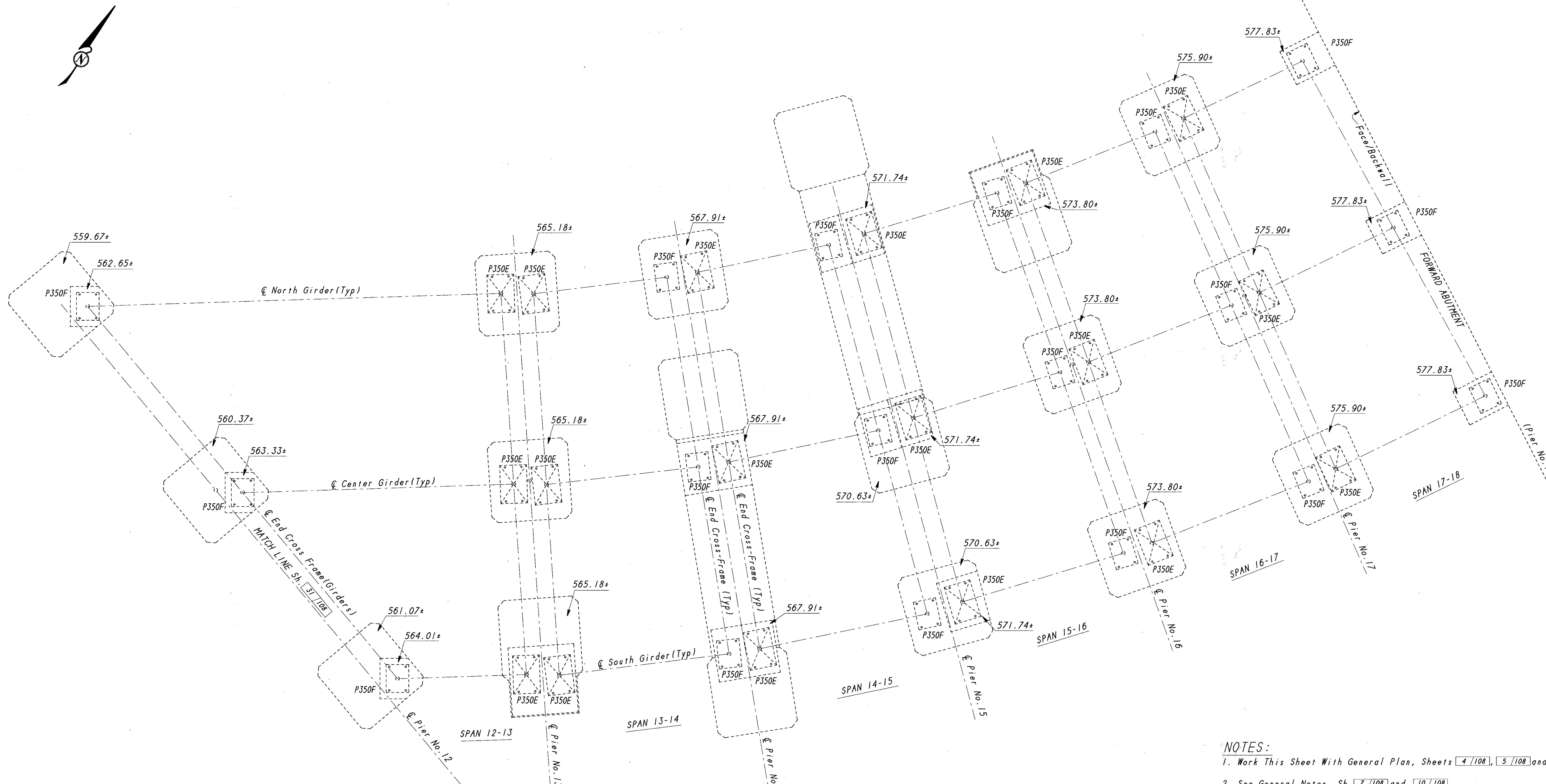
NOTES:

1. Work This Sheet With General Plan, Sheets 4/108, 5/108 and 6/108
2. Work This Sheet With Bearings Schematic Plan 3/3 Sh. 32/108
3. See General Notes, Sh. 7/108 thru Sh. 11/108.

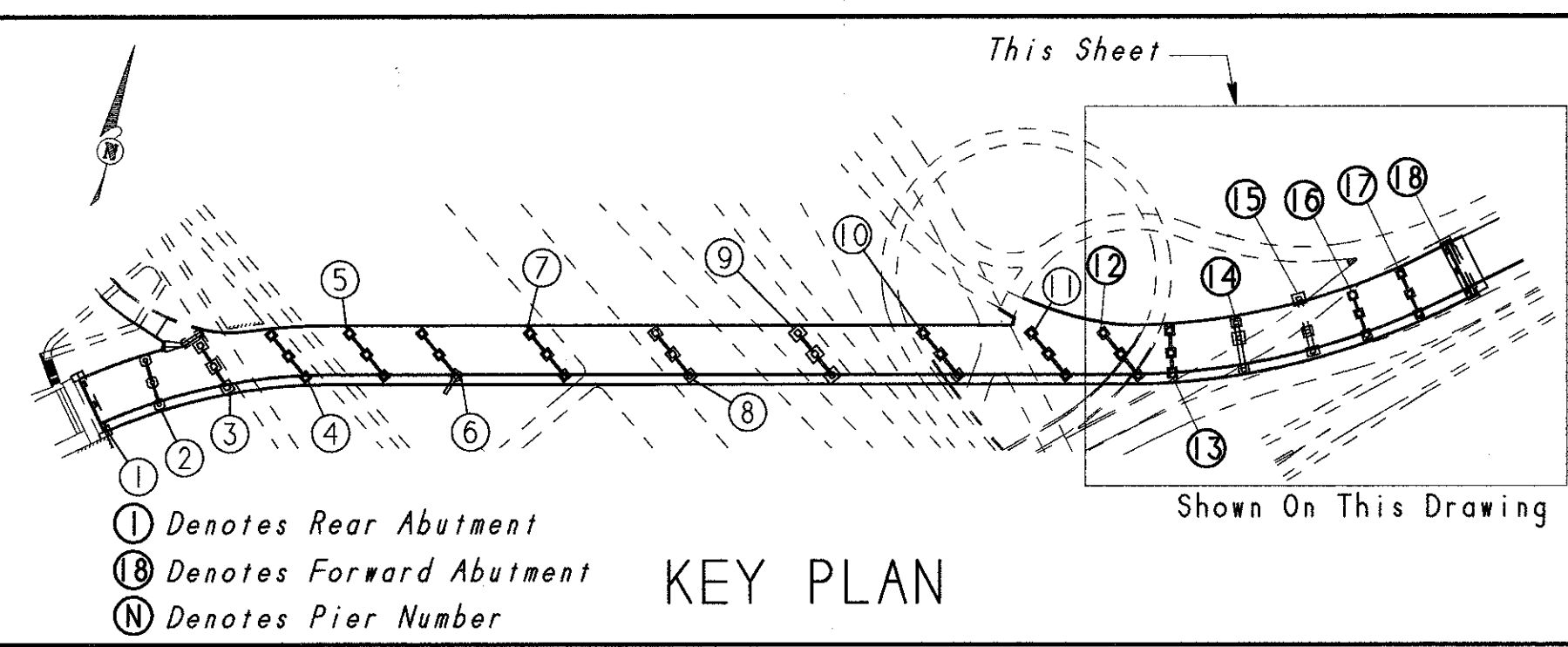


KEY PLAN

CINCINNATI		Plum, Klausmeier & Gehrm Consultants		31/108
BEARING DEVICES SCHEMATIC PLAN 2/3 PIER NO. 7 THRU PIER NO. 12				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY				Sta. 30+55.40
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
ED	ED/PLF	WDD	IEH	April 96



NOTES:
 1. Work This Sheet With General Plan, Sheets 4/108, 5/108 and 6/108
 2. See General Notes, Sh. 7/108 and 10/108.



LEGEND

- Denotes Existing Bearing.
- Denotes Elastomeric Bearing Retrofit
- 530.94± Denotes Masonry Elevation at top of pier pylon.
- P Denotes Cast Steel PEDESTAL
- P---F Denotes Existing FIXED Bearing
- P---E Denotes Existing EXPANSION Bearing

Pflum, Klausmeyer & Gehrum Consultants		32/108
BEARING DEVICES SCHEMATIC PLAN 3/3 PIER NO. 12 THRU FORWARD ABUTMENT		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

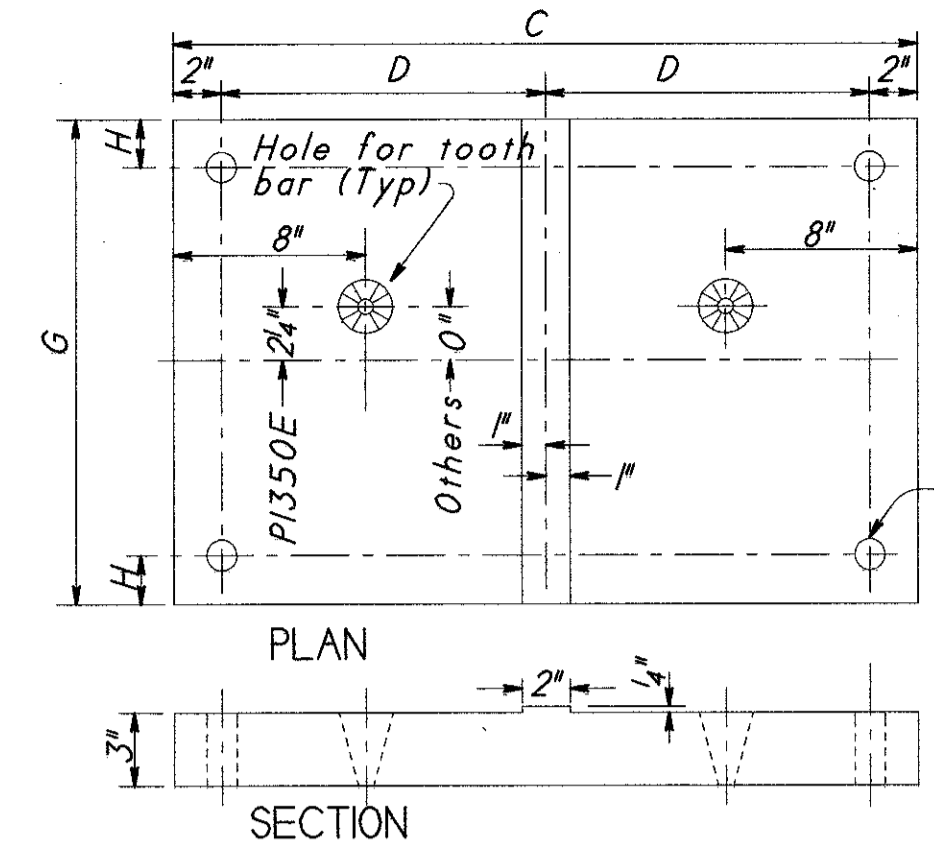
HAM-471-00.25

EXISTING MASONRY PLATE DIMENSIONS*

BEARING UNIT	C	D	G	H
P350E	3'-1"	1'-4 1/2"	2'-2"	5"
P400E	3'-4"	1'-6"	2'-2"	5"
P415E	3'-4"	1'-6"	2'-3"	5 1/2"
P460E	3'-6"	1'-7"	2'-3"	5 1/2"
PI000E	3'-11"	1'-9 1/2"	2'-10"	5"
PI350E	4'-3"	1'-11 1/2"	3'-2"	6"

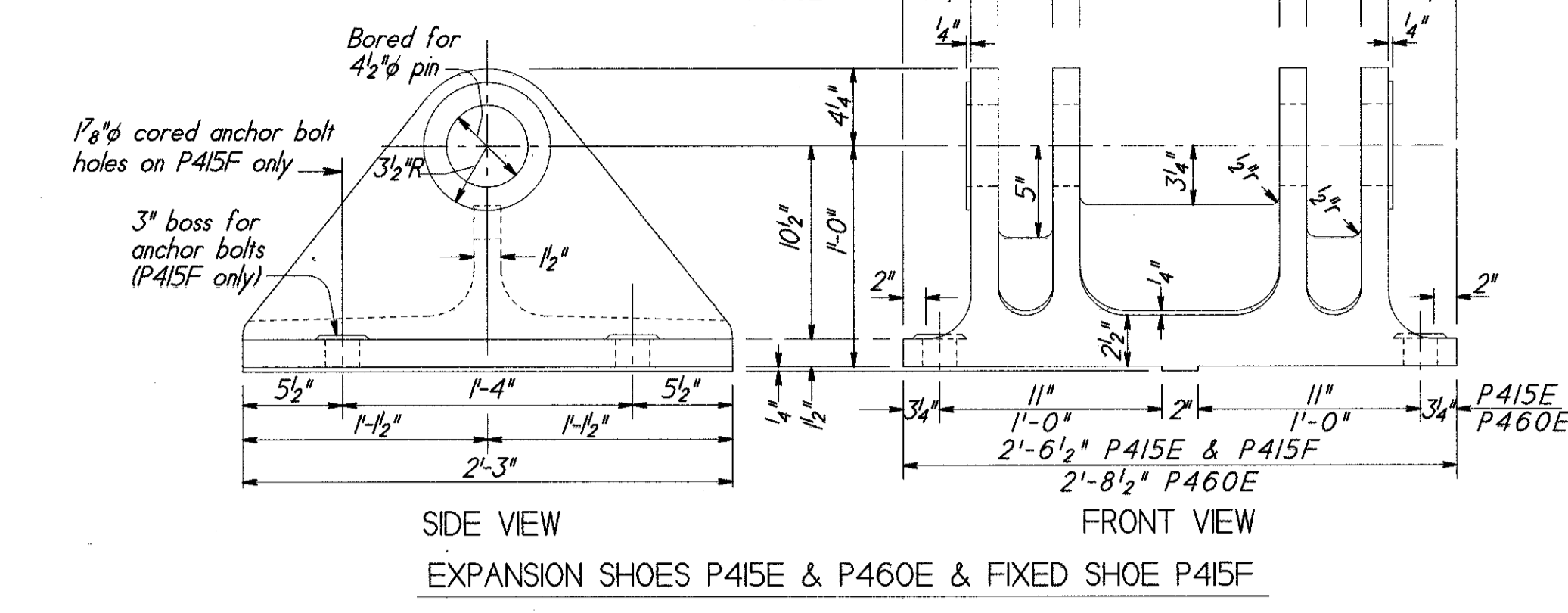
1 1/4" drilled holes for P350E & P400E
1 3/4" drilled holes for P460E, PI000E, PI350E, & P415E.

* See Sh. 361108 For Proposed Pot Bearing Masonry Plate Dimensions.



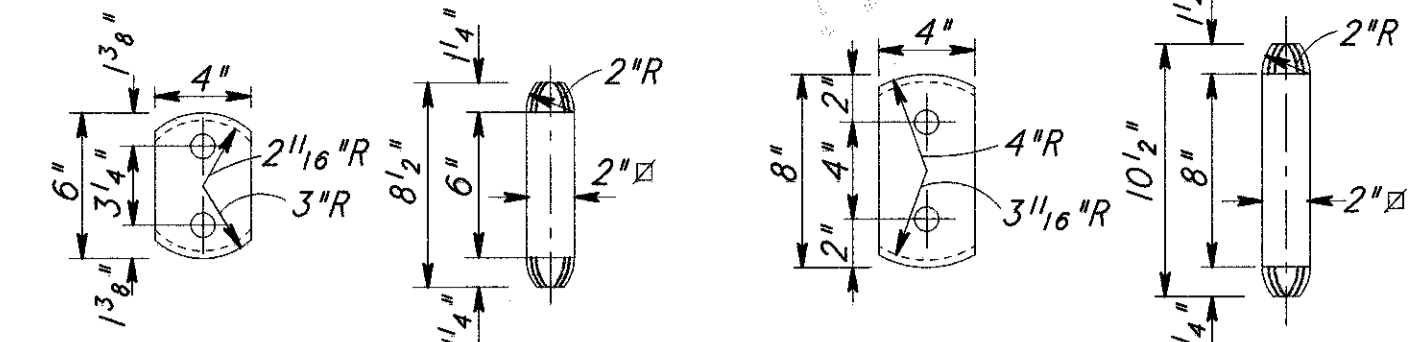
MASONRY PLATE TYPICAL DETAIL

NOTE -
No tongue and tooth bar hole in P415F
No anchr bolt holes in P415E and P460E



EXPANSION SHOES P415E & P460E & FIXED SHOE P415F

NOTE: 2"x2" slot exists in ends of rollers with tooth bars.

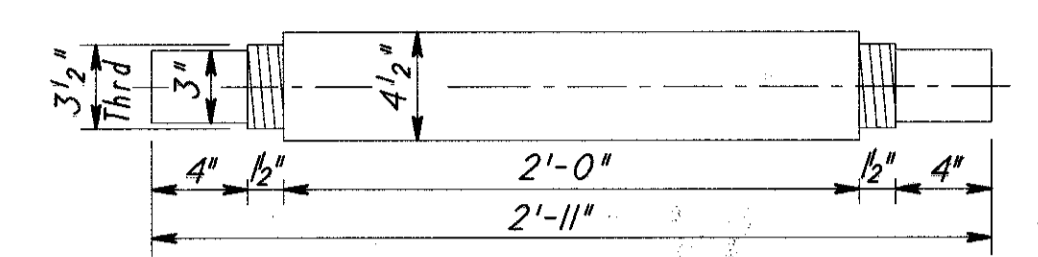


ROLLER & TOOTH BAR DETAIL
P350E, P400E, P415E, & P460E

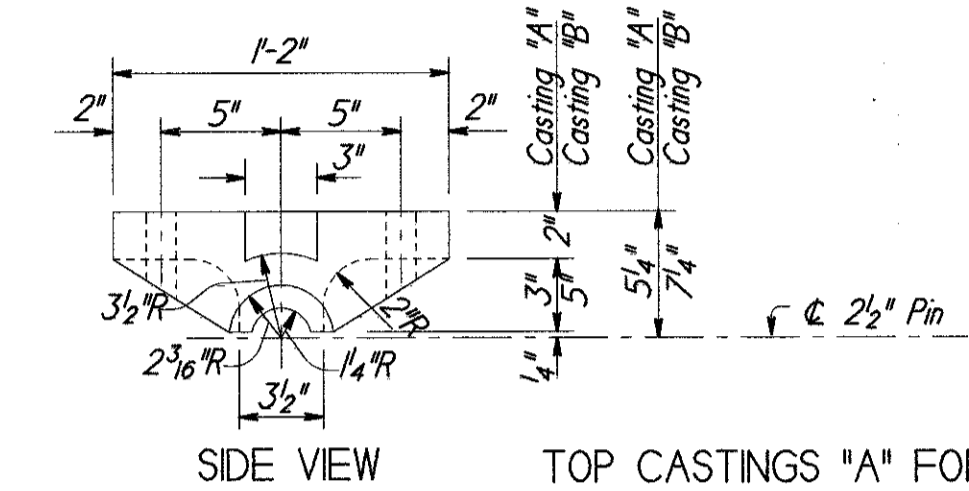
ROLLER & TOOTH BAR DETAIL
PI000E & PI350E

ROLLER SCHEDULE

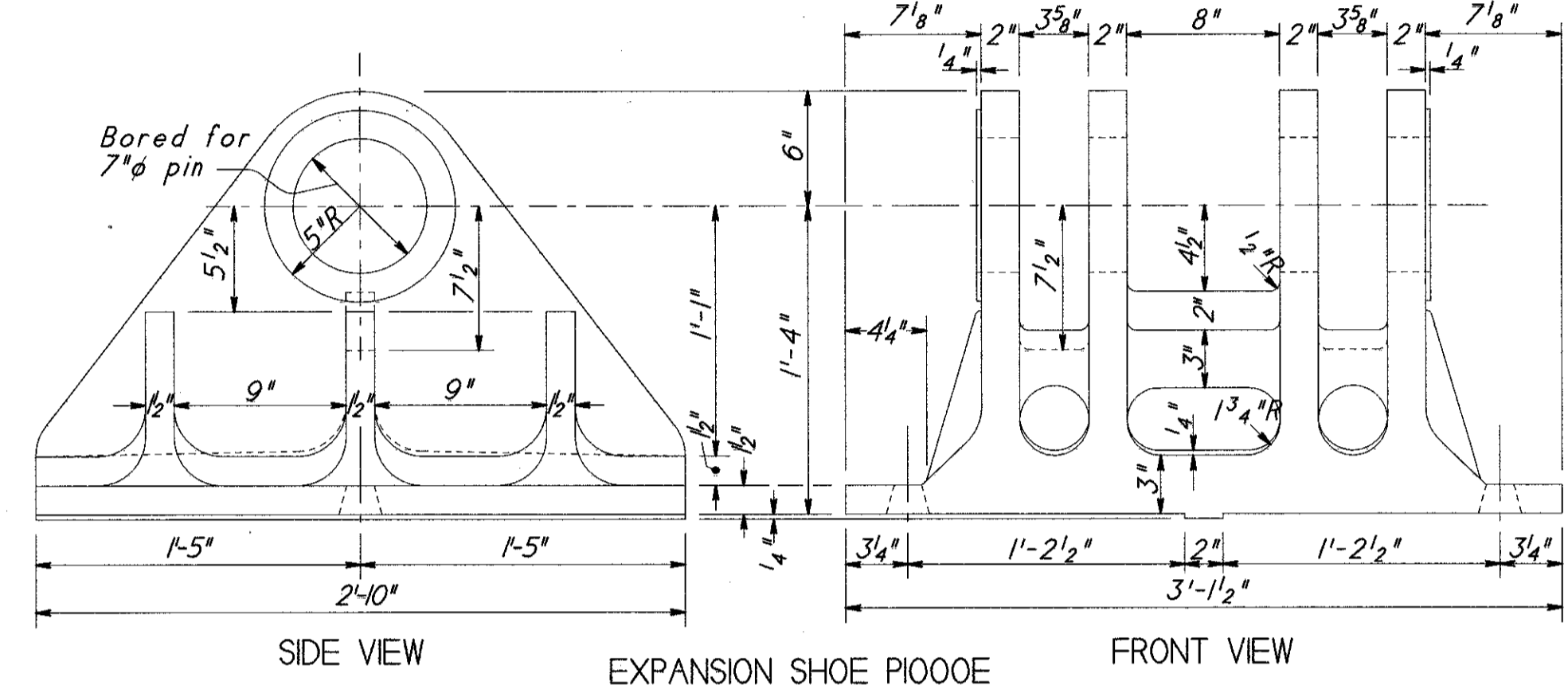
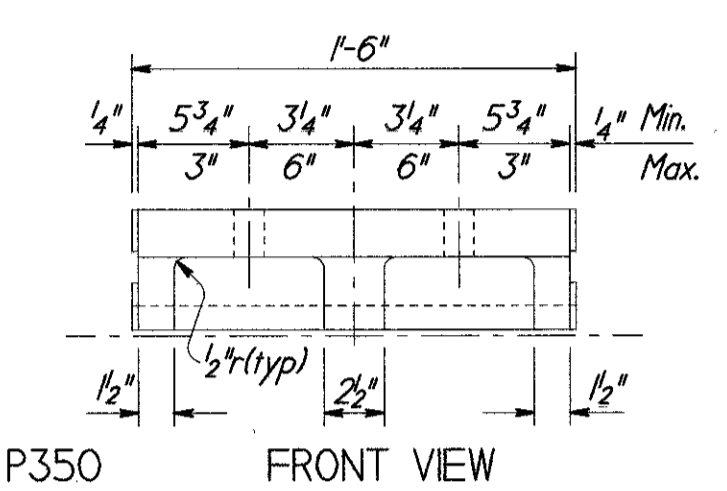
BEARING	STOCK DIAMETER	LGTH	QTY.
P350E	6"	1'-11"	5
P400E	6"	2'-2"	5
P415E	6"	2'-2"	5
P460E	6"	2'-4"	5
PI000E	8"	2'-9"	7
PI350E	8"	3'-1"	8



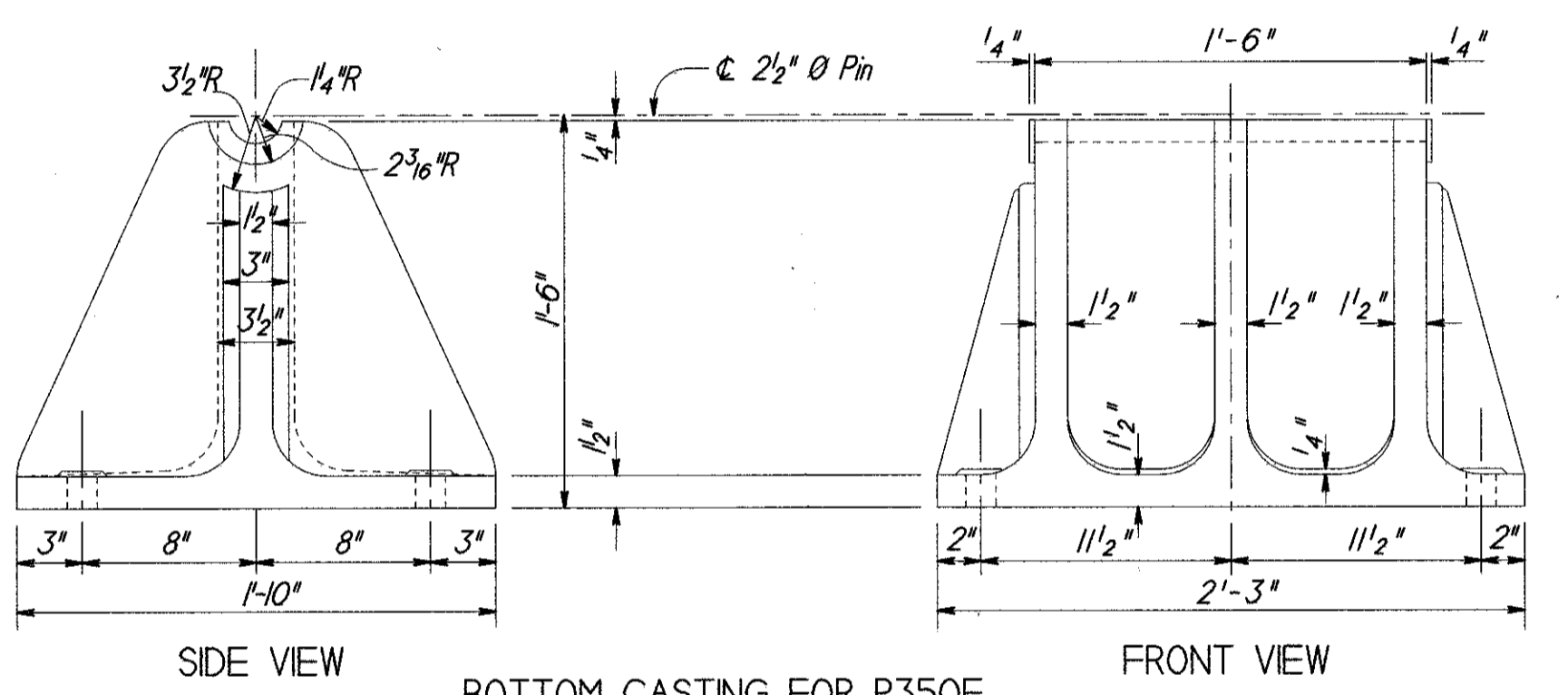
PIN & RECESSED PIN NUT DETAIL
P415E, P415F, & P460E



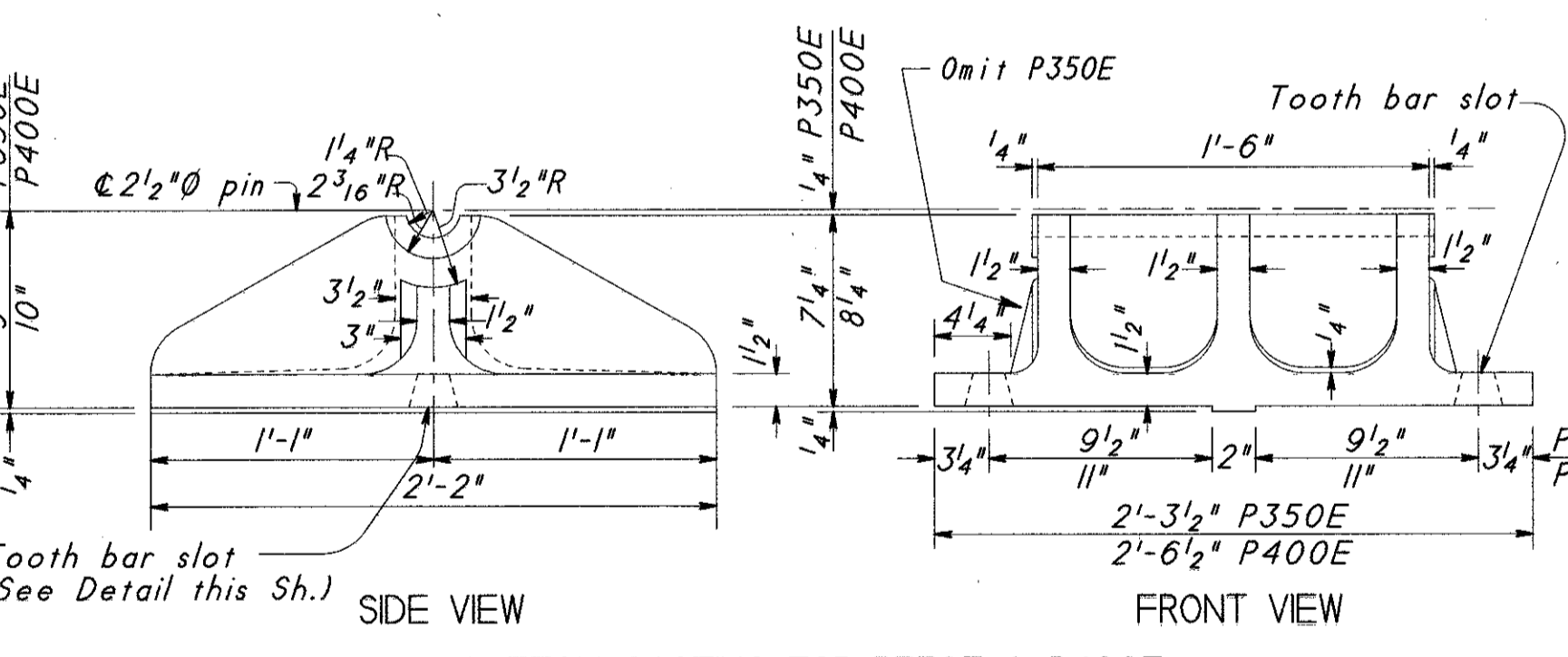
TOP CASTINGS "A" FOR P350
TOP CASTINGS "B" FOR P400



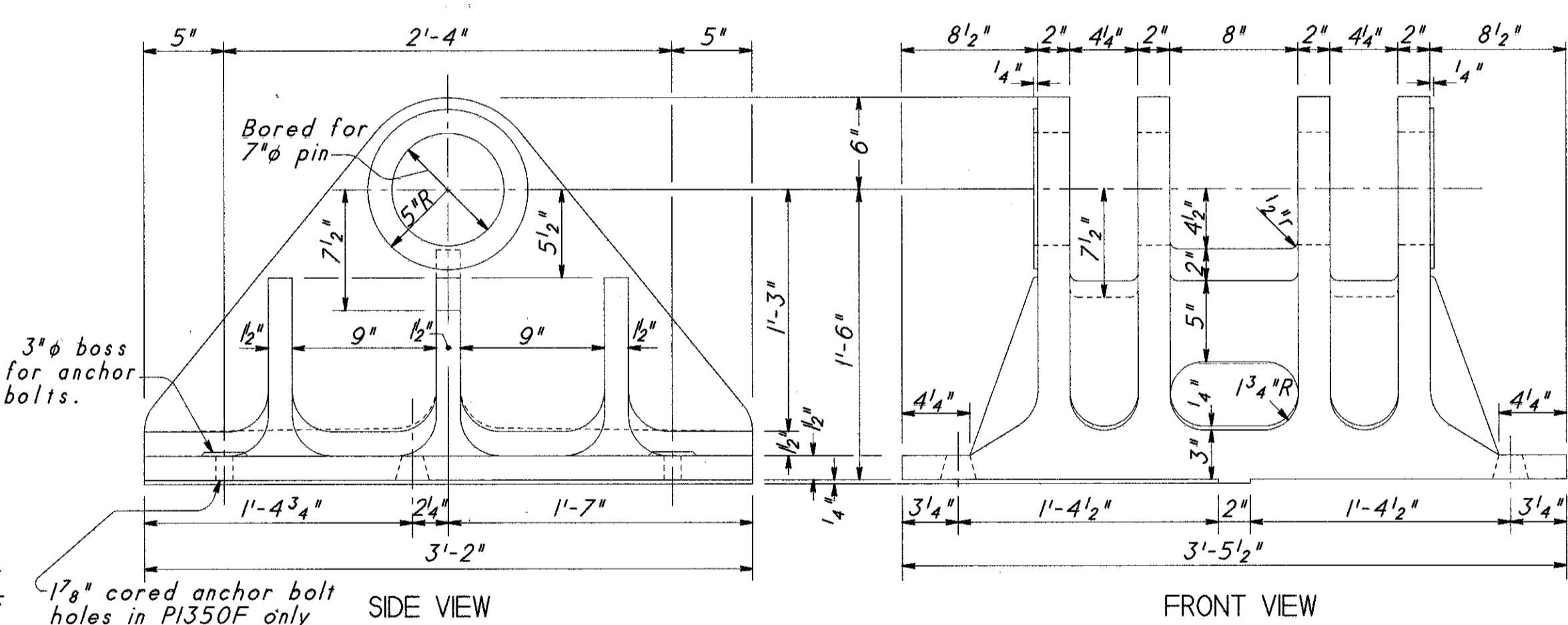
EXPANSION SHOE PI000E



BOTTOM CASTING FOR P350F

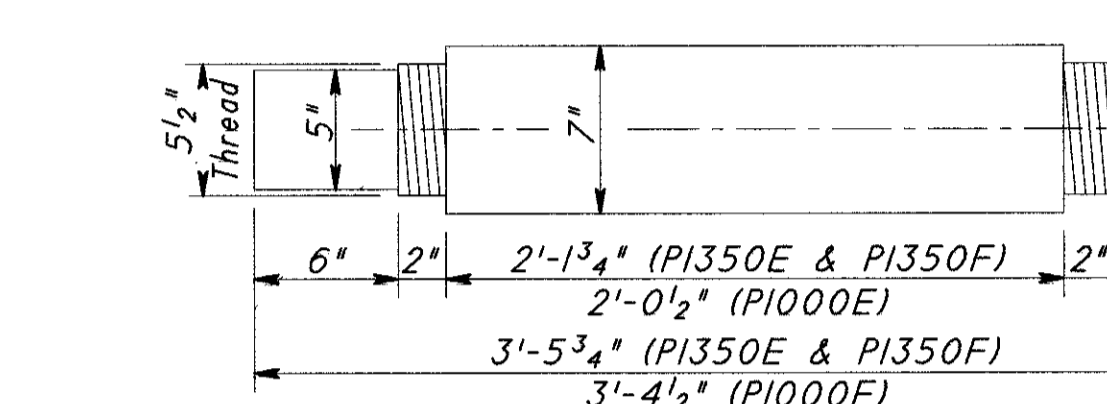


BOTTOM CASTING FOR P350E & P400E

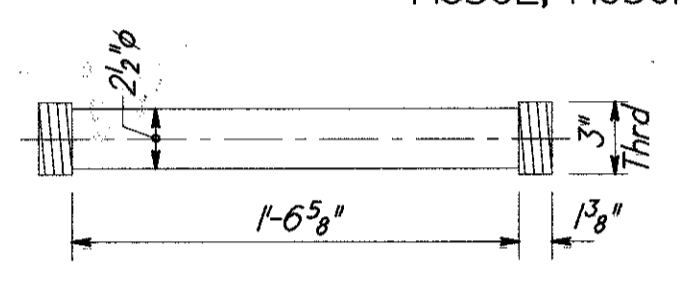


EXPANSION SHOE PI350E & FIXED SHOE PI350F

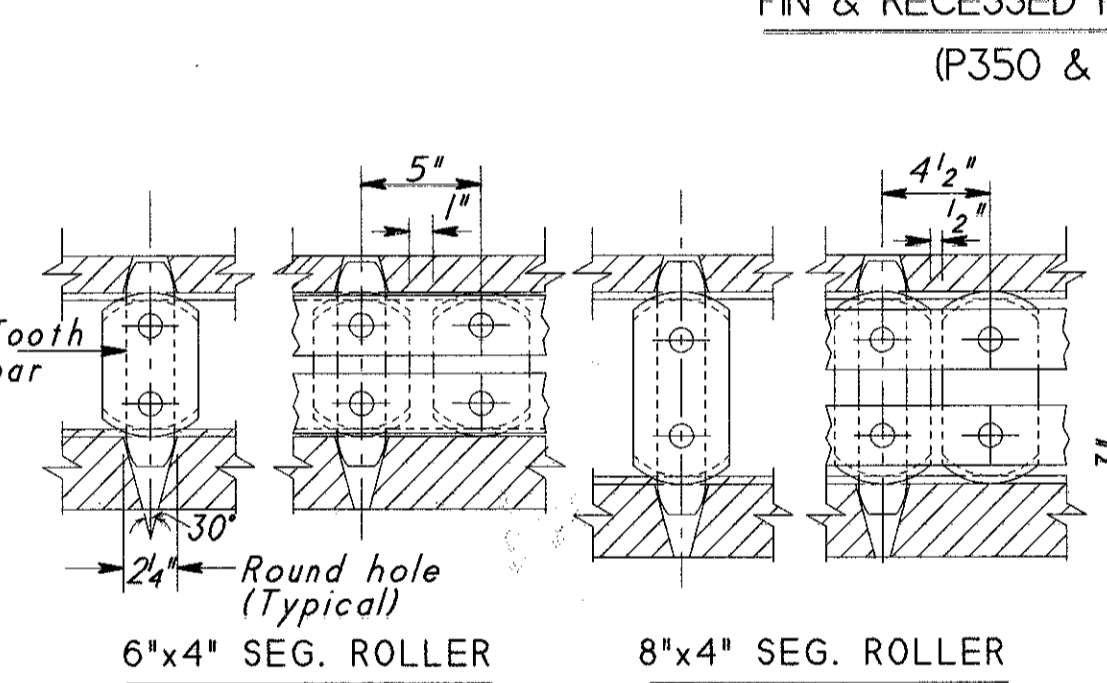
Omit tongue and tooth bar holes from PI350F
Omit anchor bolt holes from PI350E



PIN & RECESSED PIN NUT DETAIL
PI350E, PI350F & PI000E

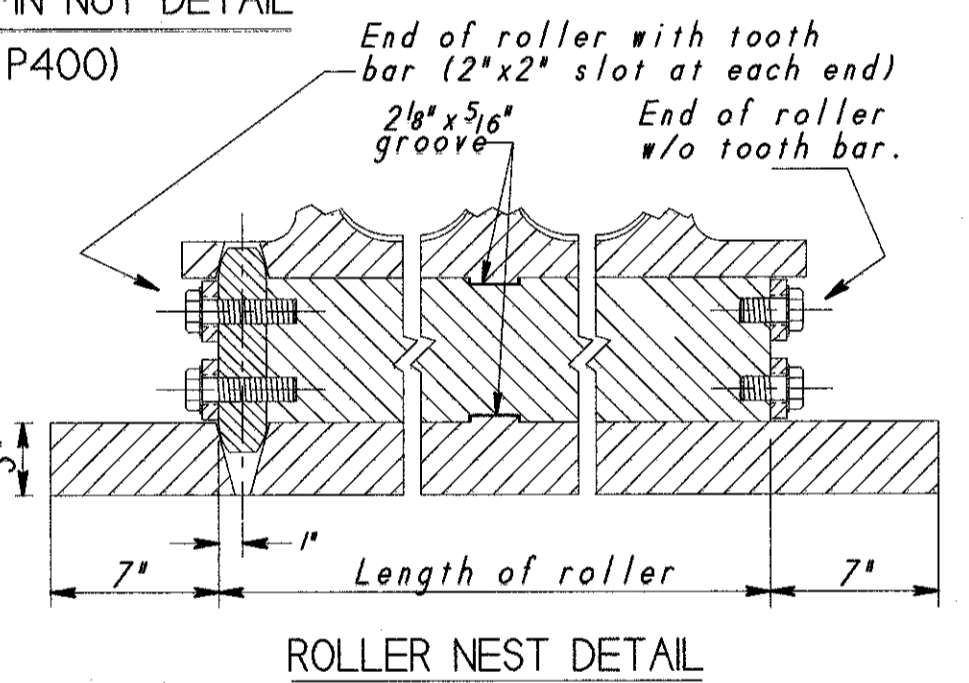


PIN & RECESSED PIN NUT DETAIL
(P350 & P400)



6"x4" SEG. ROLLER

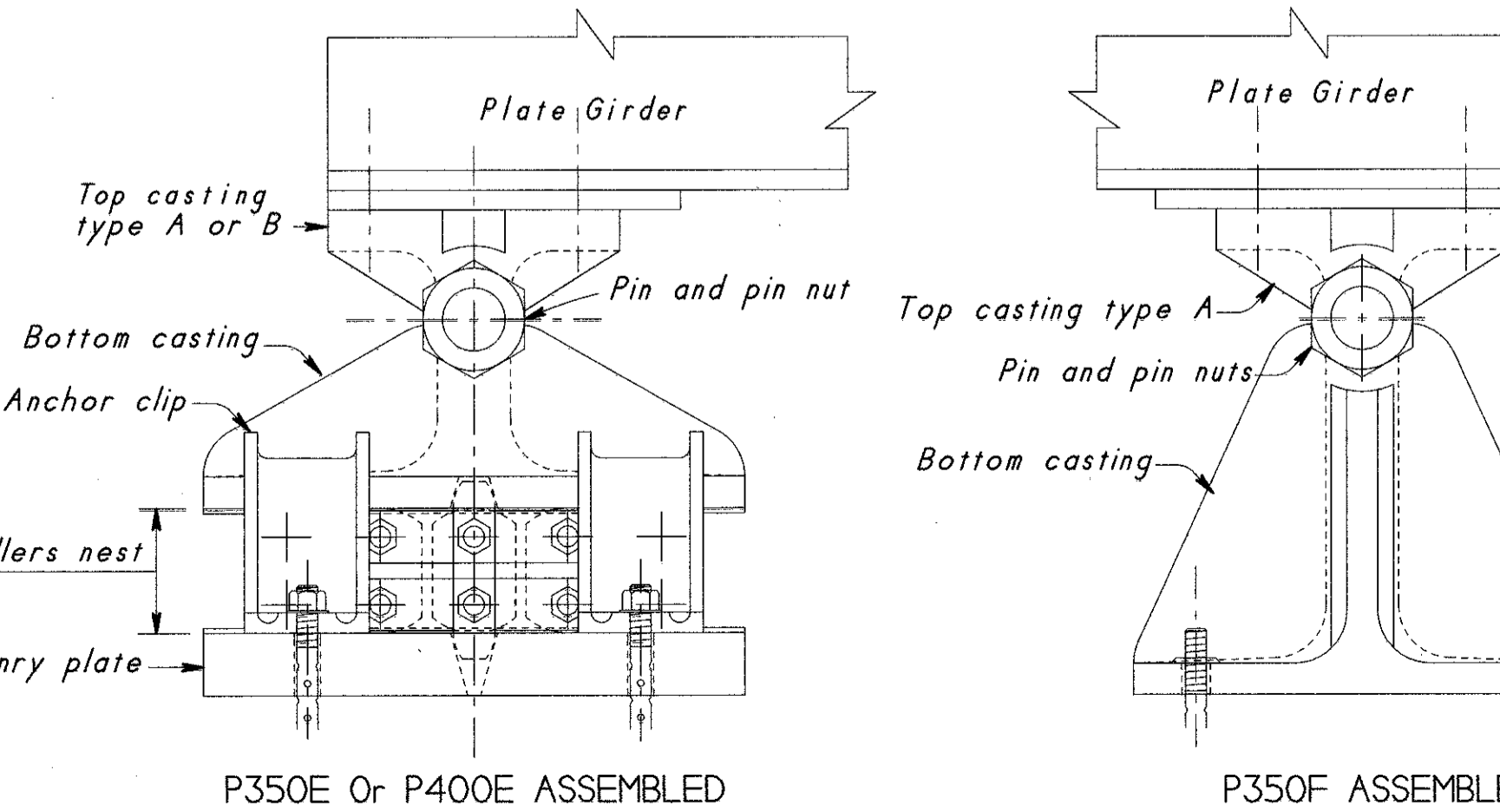
8"x4" SEG. ROLLER



ROLLER NEST DETAIL

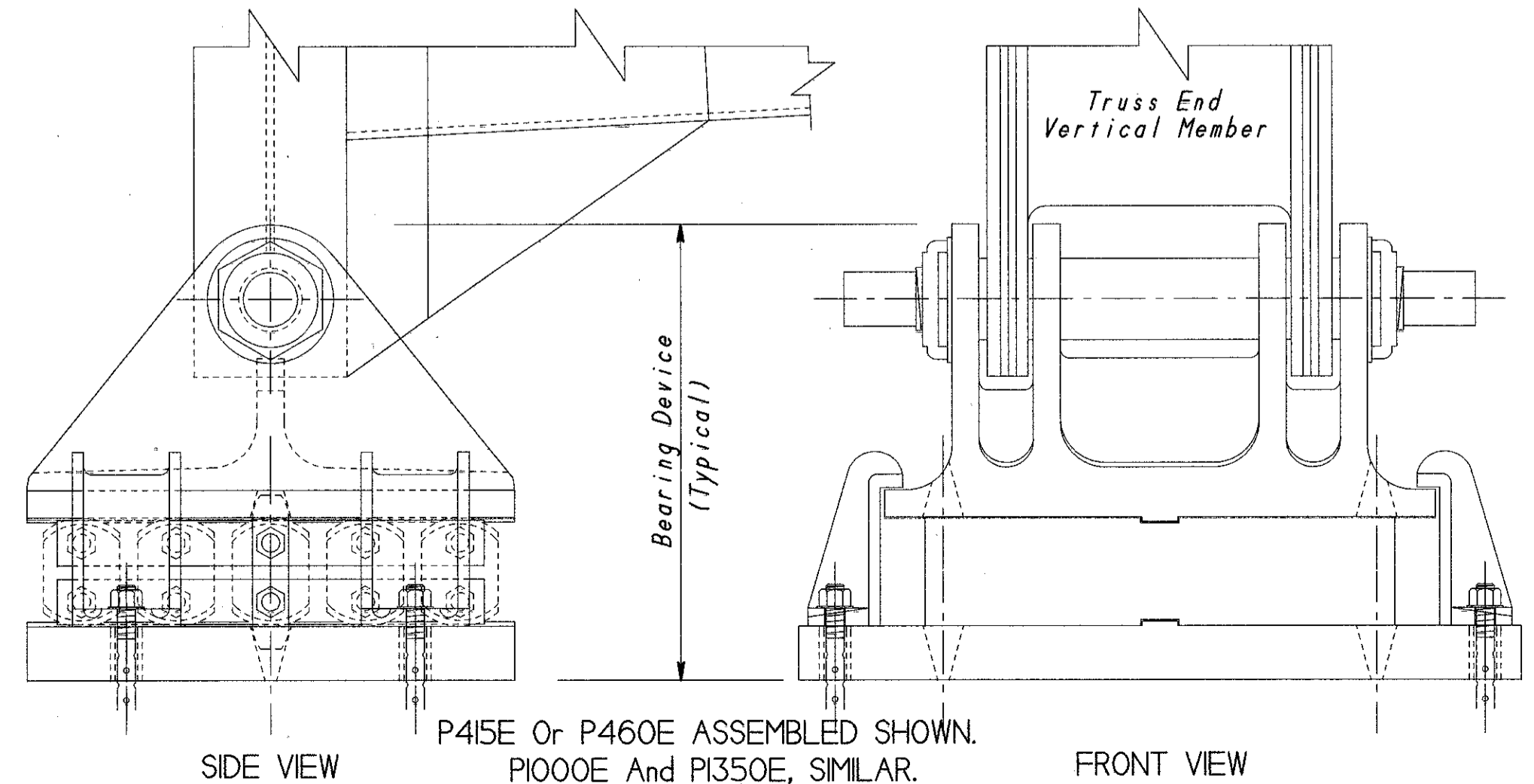
NOTE -

This drawing depicts the original plan dimensions for the bearing devices and is for information only. For additional information, the Contractor shall refer to the original plan. See Sh. 71108 for Note on Existing Structure Plans.
See Sheet 1071108 For General Note.

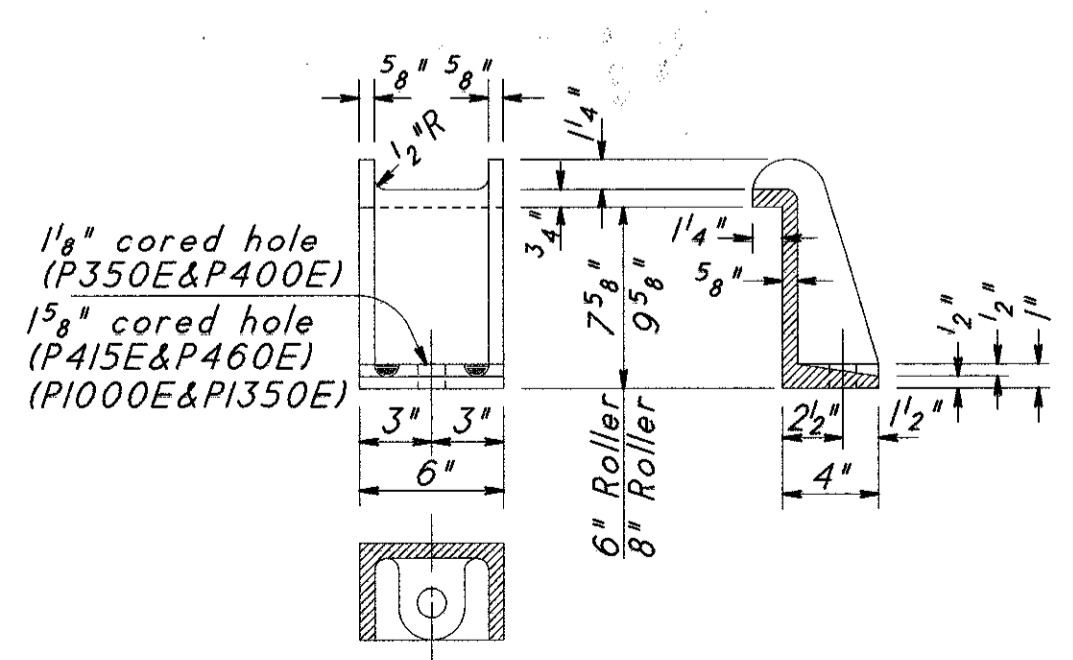


P350E Or P400E ASSEMBLED

P350F ASSEMBLED



P415E Or P460E ASSEMBLED SHOWN.
PI000E And PI350E, SIMILAR.



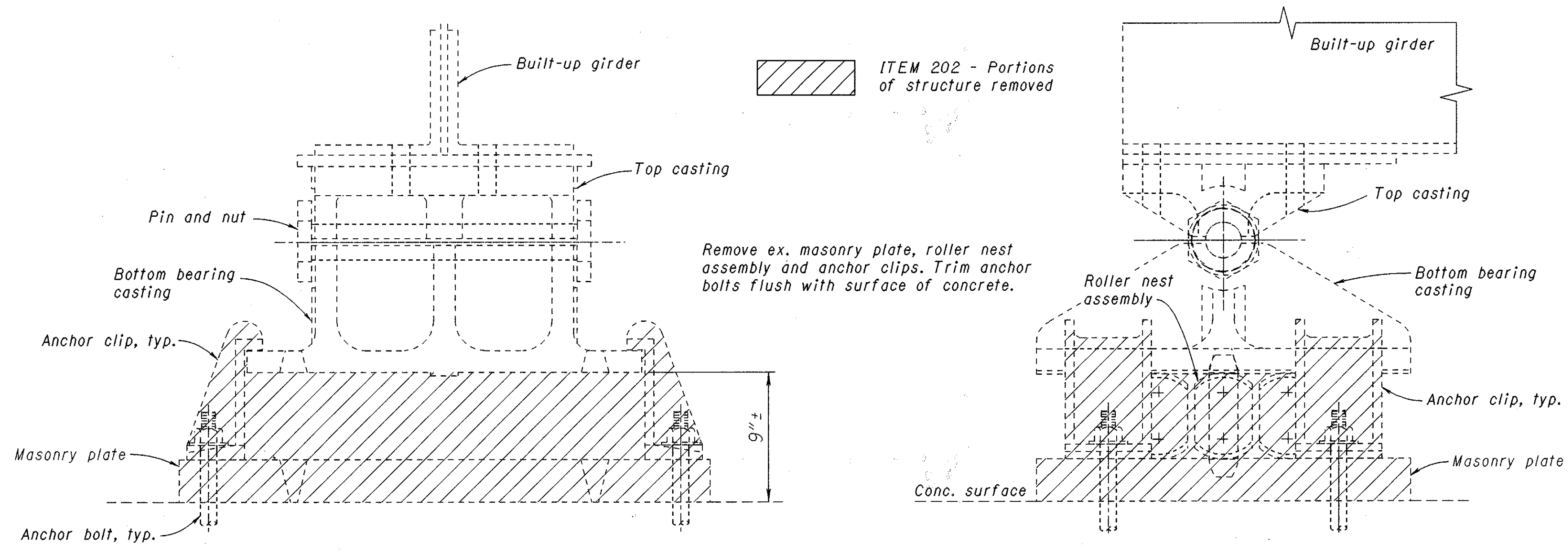
CAST STEEL ANCHOR CLIP DETAIL
(4 CLIPS AT EACH EXPANSION PEDESTAL)

Pflum, Klausmeier & Gehrmann Consultants		33 / 108
EXISTING CAST STEEL BEARING DEVICES		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	CHECKED WDD
TRACED	REVIEWED IEH April 96	DATE

CASITBRI Scale 1/666

NOTES

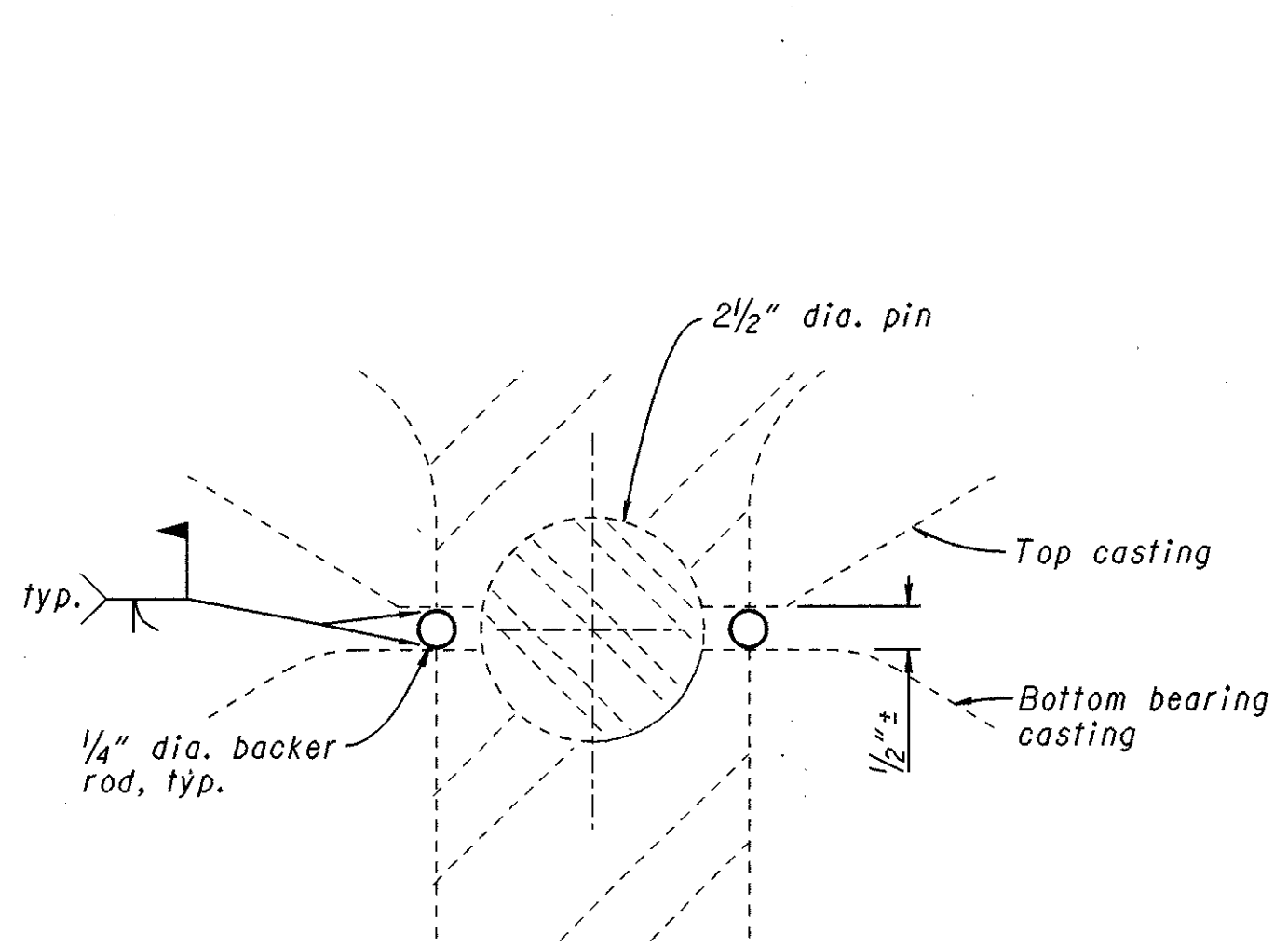
- For bearing locations, see Sheets No. 30/108 and 32/108.
- Material requirements: Steel load plate: ASTM A36
Steel laminates: ASTM A36
Elastomer: 50 Durometer Neoprene, Grade 3
- Welding shall be controlled so that the plate temperature at the elastomer bonded surface does not exceed 300°F as determined by the use of pyrometric sticks or other temperature monitoring devices.
- ELASTOMERIC BEARINGS shall comply with CMS 516 and Articles 18.2.5 through 18.2.8 of Section 18, Bearing Devices, Division 11, Construction of the AASHTO standard Specification for highway Bridges. Bearings shall be Grade 3, 50 Durometer elastomer, and shall be subjected to the load testing requirements corresponding to Design Method A. Testing shall be included in the unit price bid for the bearings, each.
- The steel load plate shall be ASTM A36 steel and be cleaned and coated with IZEU paint system. Surface preparation and priming shall be done in the shop and included in the price bid for the bearings. Field coats shall be included in the price bid for painting the main structural steel.
- Proposed work includes: After removal of the existing concrete deck and end dams, jack and support the superstructure in accordance with plan notes. Remove the existing rollers and masonry plate and remove anchor bolts flush with the concrete surface. Repair concrete seat area as necessary to provide a solid and uniform surface using a trowelable epoxy mortar. Grind seat as necessary for a smooth level surface. Install new elastomeric bearings. Lower girders to permanent position.
- BASIS OF PAYMENT: Payment shall be made under Item 516, 29 EACH, "Elastomeric bearing with internal laminates and load plate (Neoprene), 2'-2 1/2" X 2'-1" X 6/8" laminated elastomeric pad and 2'-3 1/2" X 2'-2" X 3" steel load plate, as per plan" and 1 EACH, "Elastomeric bearing with internal laminates and load plate (Neoprene), 2'-2 1/2" X 2'-1" X 6/8" laminated elastomeric pad and 2'-6 1/2" X 2'-2" X 3" steel load plate, as per plan" and shall include the removal of portions of the existing bearing, cleaning the existing bearing casting, furnishing and installing the new bearing, preparation of the concrete surface and any additional work that is necessary to properly install the bearing.



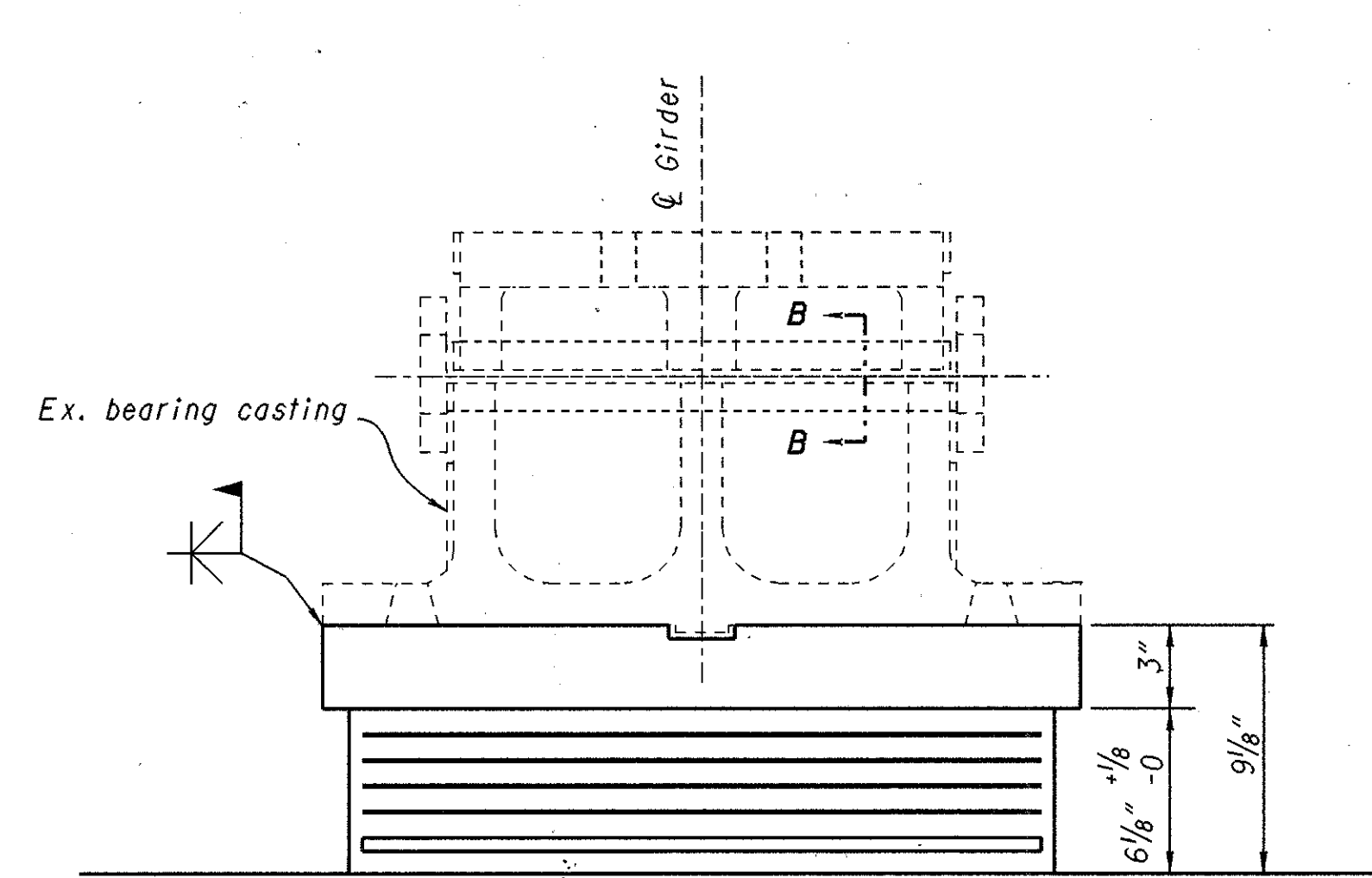
FRONT VIEW

SIDE VIEW

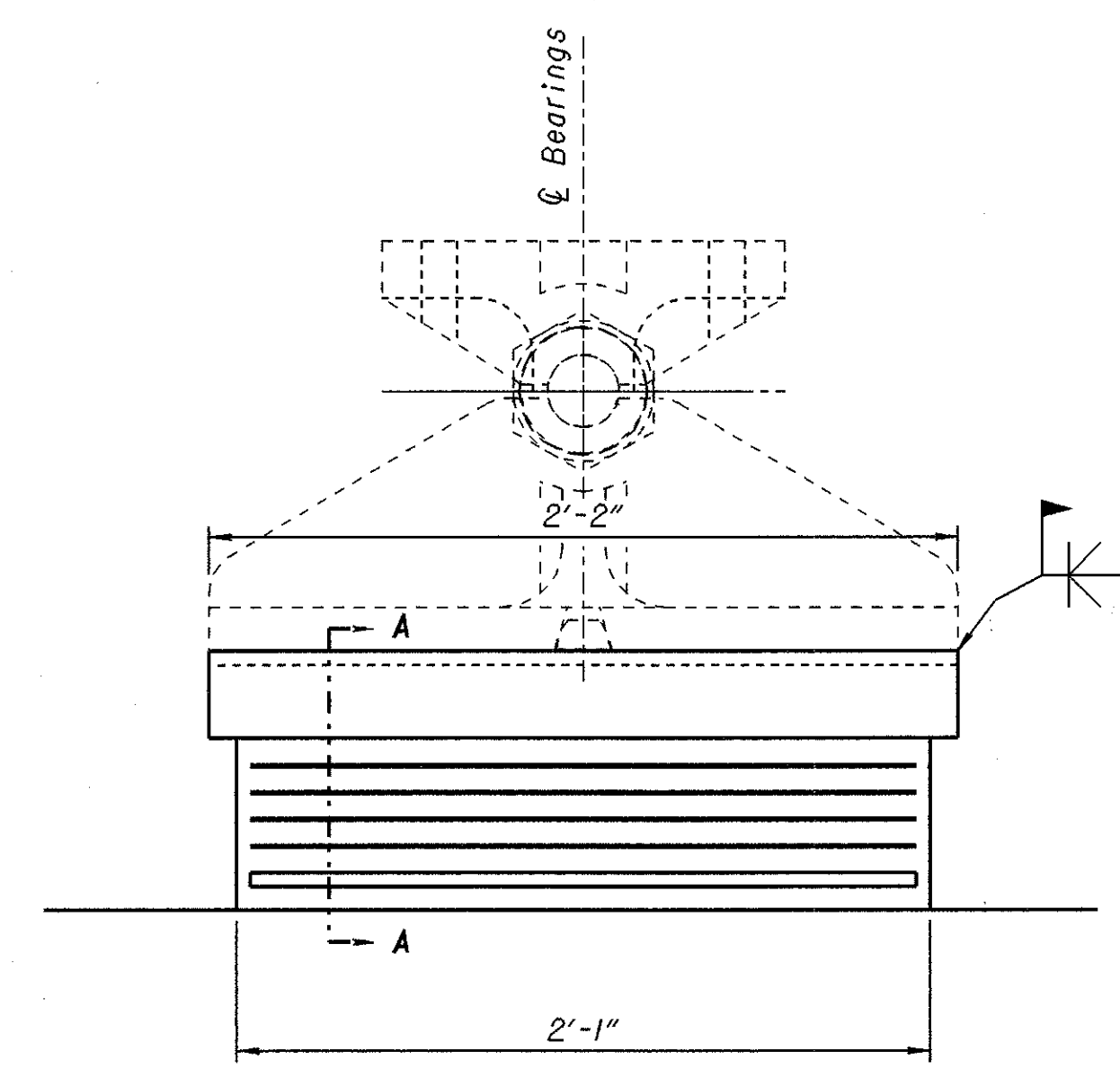
REMOVAL DETAILS



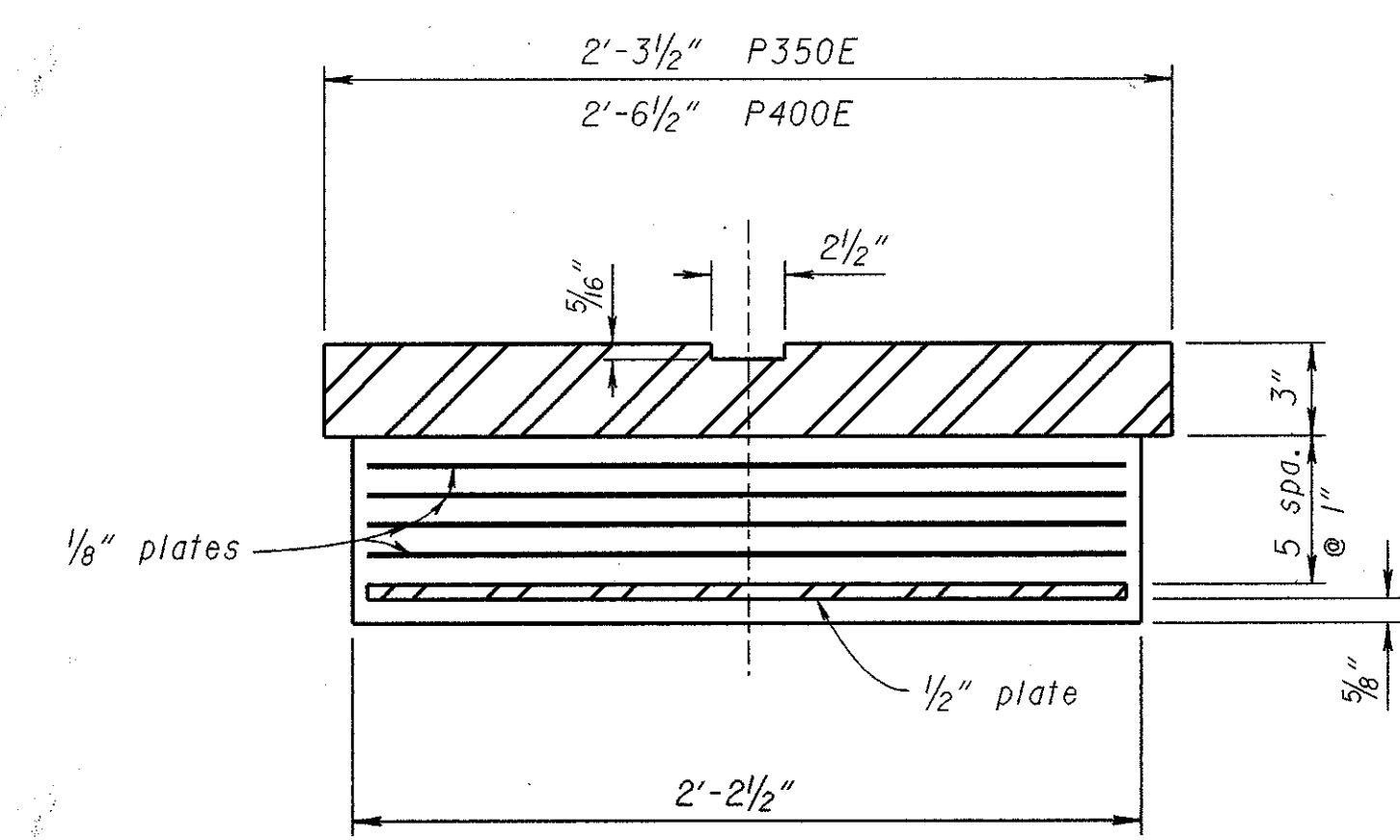
SECTION B-B



FRONT VIEW



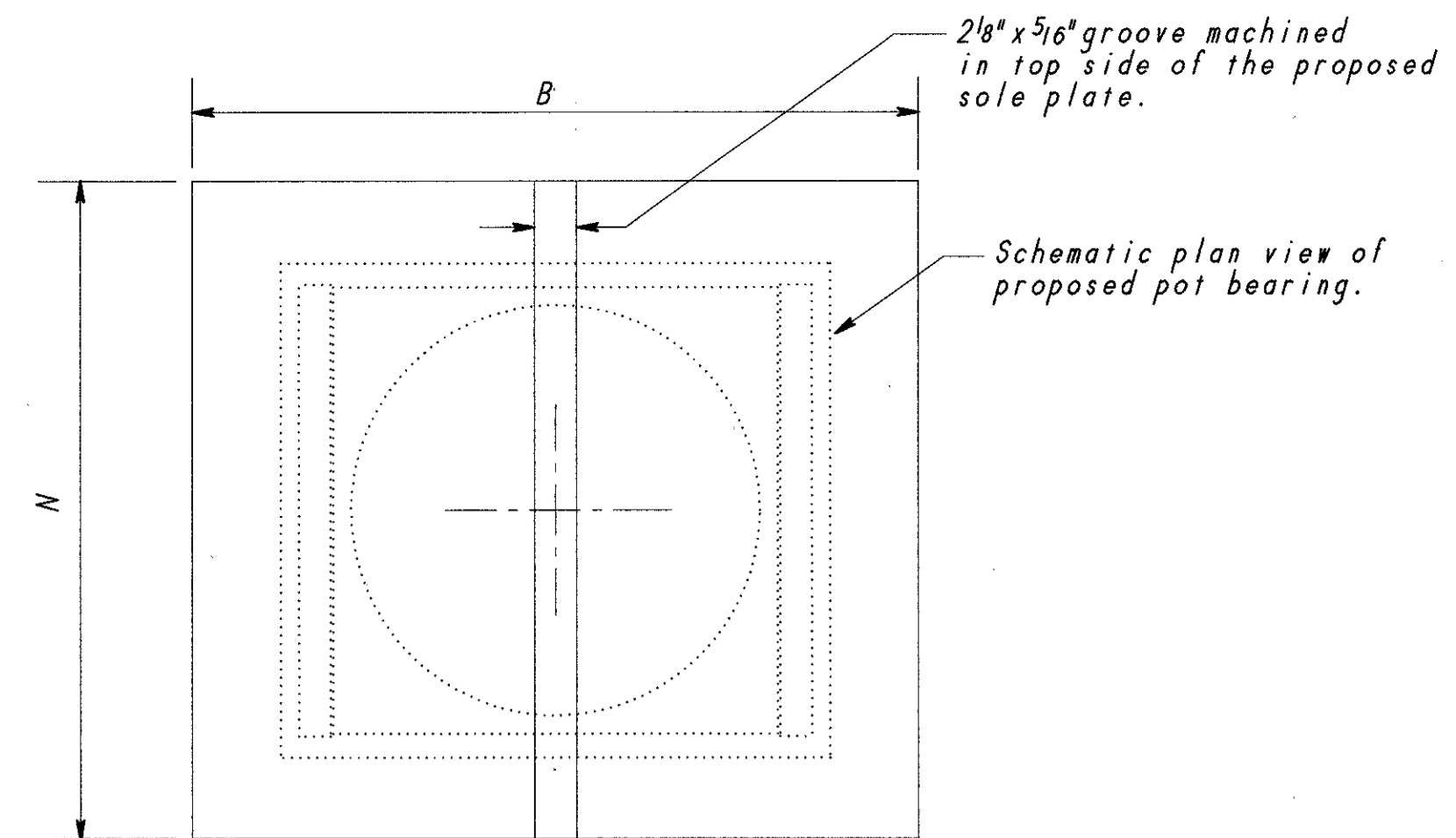
SIDE VIEW



SECTION A-A

P350E AND P400E BEARING RETROFIT DETAILS

OHIO DEPARTMENT OF TRANSPORTATION					
DISTRICT 8 PRODUCTION 34/108					
ELASTOMERIC BEARING					
RETROFIT DETAILS					
BRIDGE NO. HAM-471-0025					
COLUMBIA PARKWAY VIADUCT					
DESIGNED	DRAWN	CHECKED	REVIEWED	DATE	REVISIONS
MLM	MLM BJF	MLM	RLE	8-27-96	



TYPICAL ADDITIONAL SOLE PLATE

(See Table A this sheet for dimensions)

TABLE A - POT BEARING DESIGN PARAMETERS

STRUCTURE DESCRIPTION	PIER	MAXIMUM COMPUTED VERTICAL LOAD (KIP)	MINIMUM COMPUTED VERTICAL LOAD (KIP)	MAXIMUM TRANSVERSE HORIZONTAL LOAD (KIP)	-30°F to +120°F TEMP. RANGE ANTICIPATED TRAVEL DIST. (AS PER 516 COLD CLIMATE)	POT BEARING ANTICIPATED ROTATION (RADIAN)	MASONRY PLATE PROPOSED DIMENSIONS					1 1/2" DIAMETER ANCHOR BOLTS*		PROPOSED SOLE PLATE TO BE WELDED TO EXIST. CAST STEEL SHOE			PROPOSED POT BEARING DESIGNATION	DESIGN VERTICAL FORCE	DESIGN HORIZONTAL FORCE	DESIGN TRAVEL DISTANCE	POT BEARING EFFECTIVE DEPTH #	EXISTING ROLLER BEARING TO BE MODIFIED
							P	A	B	W	L	LENGTH	No.	T	B	N						
TWO SPAN TRUSS (REAR)	No. 5	242	122	25	2 1/2"	0.0044	3"	1'-4"	3'-0"	3'-5"	1'-10"	18"	4	2 3/8"	2'-4"	2'-0"	PB300E	300K	30K	3'	3.12"	P415E
	No. 6	974	570	98	1 1/2"	0.0052	3"	2'-0"	3'-7"	4'-0"	2'-6"	21"	4	3 3/8"	3'-0"	2'-10"	PB1000E	1000K	100K	2'	4.89"	P1000E
THREE SPAN TRUSS (MAIN)	No. 7	428	197	43	3 3/4"	0.0049	3"	1'-4"	3'-2"	3'-7"	1'-10"	18"	4	2 3/8"	2'-6"	2'-0"	PB500E	500K	50K	4'	3.66"	P460E
	No. 8	1077	753	107	2"	0.0038	3"	2'-2"	3'-11"	4'-4"	2'-8"	21"	4	2 3/8"	3'-0"	2'-10"	PB1200E	1200K	120K	2'	5.21"	P1350E
TWO SPAN TRUSS (FORWARD)	No. 10	428	197	43	1 3/4"	0.0040	3"	1'-4"	3'-2"	3'-7"	1'-10"	18"	4	2 3/8"	2'-6"	2'-0"	PB500E	500K	50K	2'	3.66"	P460E
	No. 11	974	570	98	1 1/2"	0.0052	3"	2'-0"	3'-7"	4'-0"	2'-6"	21"	4	3 3/8"	3'-0"	2'-10"	PB1000E	1000K	100K	2'	4.89"	P1000E
	No. 12	242	122	25	2 1/2"	0.0044	3"	1'-4"	3'-0"	3'-5"	1'-10"	18"	4	2 3/8"	2'-4"	2'-0"	PB300E	300K	30K	3'	3.12"	P415E

* Based On Pot Bearing Series PMG As Manufactured By D.S.Brown Co. (See Note 3, this Sh.)

The existing roller bearing shall be released off the truss main support member by lifting the truss 1/4" maximum off the pier. The roller nest and the masonry plate shall be removed. The pin holding the cast steel shoe shall be released and the cast steel shoe removed. The bored hole in the truss support member shall be cleaned, lubricated (graphite), and protected. The existing pin shall be cleaned,

The cast steel shoe shall be cleaned, the hole to receive the pin shall be lubricated, and the shoe underside face shall be ground to flatness class A. When re-installed, the bottom face of the cast steel shoe and the pin axis shall be parallel w/o tolerance. All labor, material, equipment, tools, and incidentals necessary to complete the work described above, shall be included with Item 516, Steel Pot Bearing, As Per Plan, (___Kips), for payment, except that jacking and temporary support shall be included with Item 516, Jacking And Temporary Support Of Superstructure, As Per Plan, for payment.

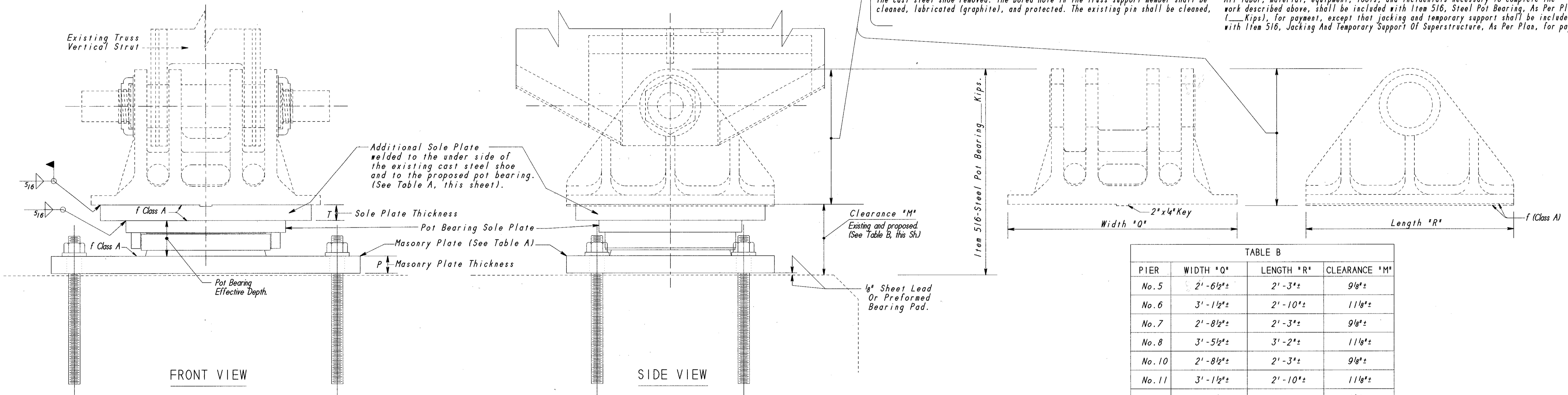


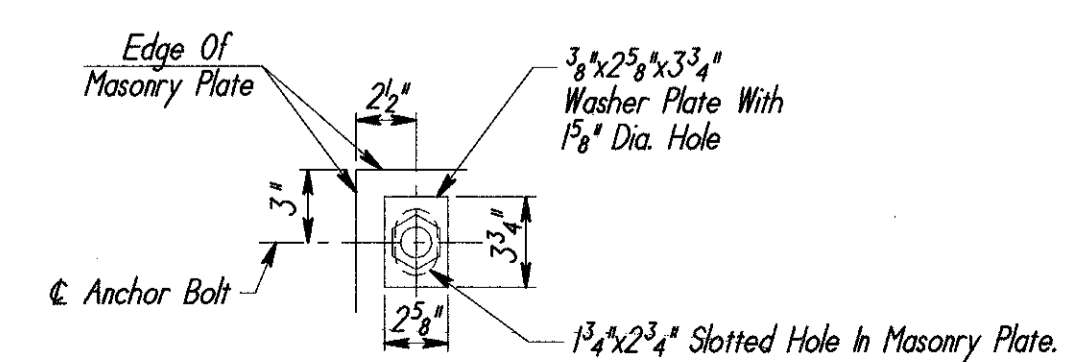
TABLE B

PIER	WIDTH "Q"	LENGTH "R"	CLEARANCE "M"
No. 5	2'-6 1/2" ±	2'-3" ±	9/8" ±
No. 6	3'-1 1/2" ±	2'-10" ±	1 1/8" ±
No. 7	2'-8 1/2" ±	2'-3" ±	9/8" ±
No. 8	3'-5 1/2" ±	3'-2" ±	1 1/8" ±
No. 10	2'-8 1/2" ±	2'-3" ±	9/8" ±
No. 11	3'-1 1/2" ±	2'-10" ±	1 1/8" ±
No. 12	2'-6 1/2" ±	2'-3" ±	9/8" ±

* The existing anchor bolts shall be protected during bearing device removal, and be incorporated into the new work. If damaged due to the Contractor's removal operation, bolt shall be replaced with new as per table A this sheet at his cost. If deemed unusable by the Engineer due to any reason, bolt shall be removed, and replaced with new as per table A this sheet, and the cost of the work be included with Item 516, Steel Pot Bearing, As Per Plan, (___Kips), for payment.

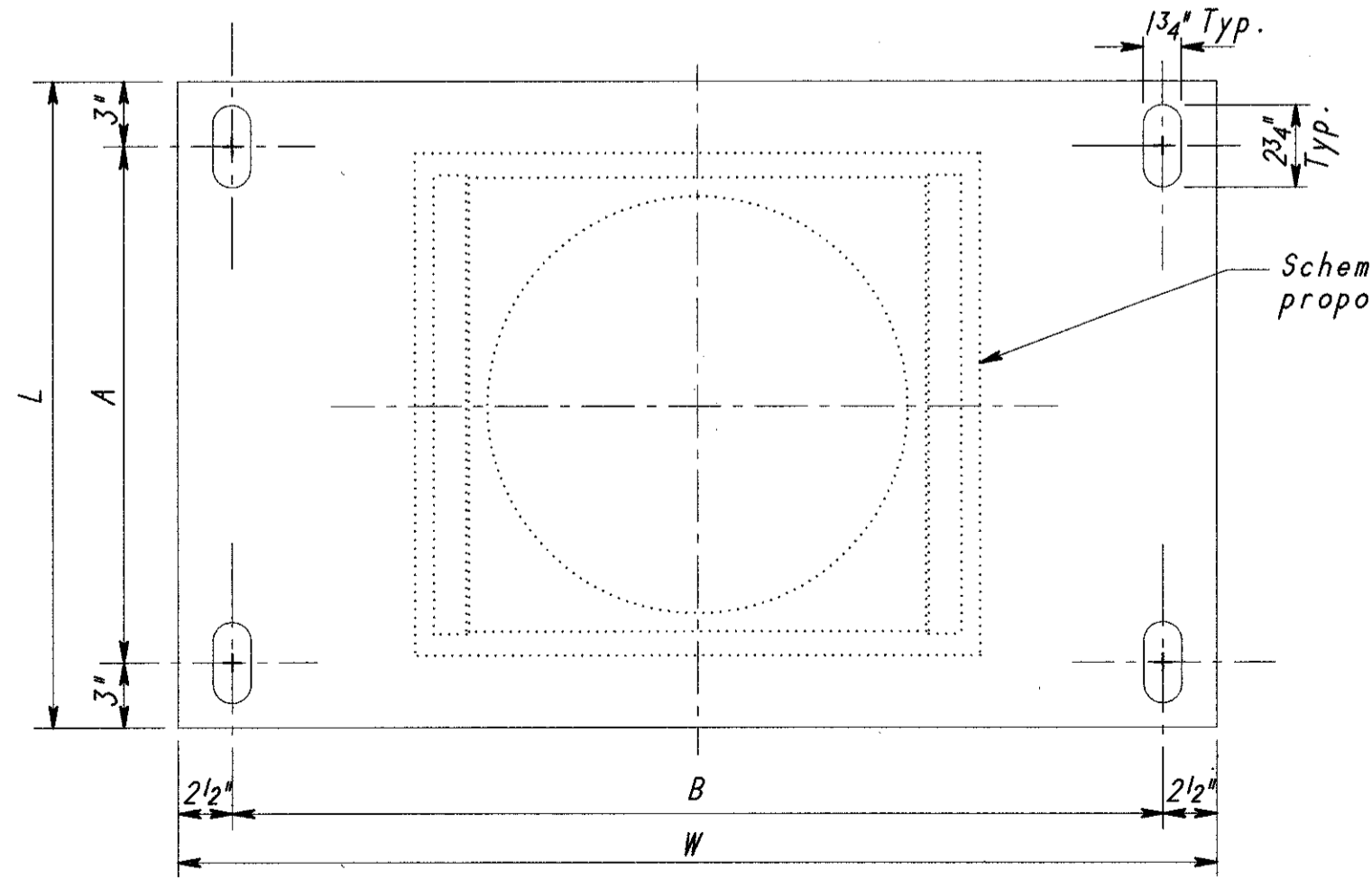
NOTES -

- 1.- Work this sheet with Sheets 30/108 thru Sh. 32/108 ; and Sh. 37/108 .
- 2.- See GENERAL NOTES, Sh. 8/108 and Sh. 9/108 for Item 516-STEEL POT BEARINGS, AS PER PLAN, TYPE GUIDED.
- 3.- Bearing Device Shall Be Of The Uni-Directional Pot Bearing Series PMG As Manufactured By The D.S. BROWN COMPANY, 300 East Cherry St., North Baltimore, Ohio 45872-0158. Or, Laterally-Restricted Uni-Ton Pot Bearing Of The Series UBLR As Manufactured By AUGUSTA IRON & STEEL WORKS, INC., 306 11th Street Augusta, Georgia, 30901. Or Approved Equal.
- 3.- Masonry Plate Shall Be Provided With Slotted Holes As Shown Below.



TYPICAL POT BEARING DEVICE

(TYPE GUIDED)



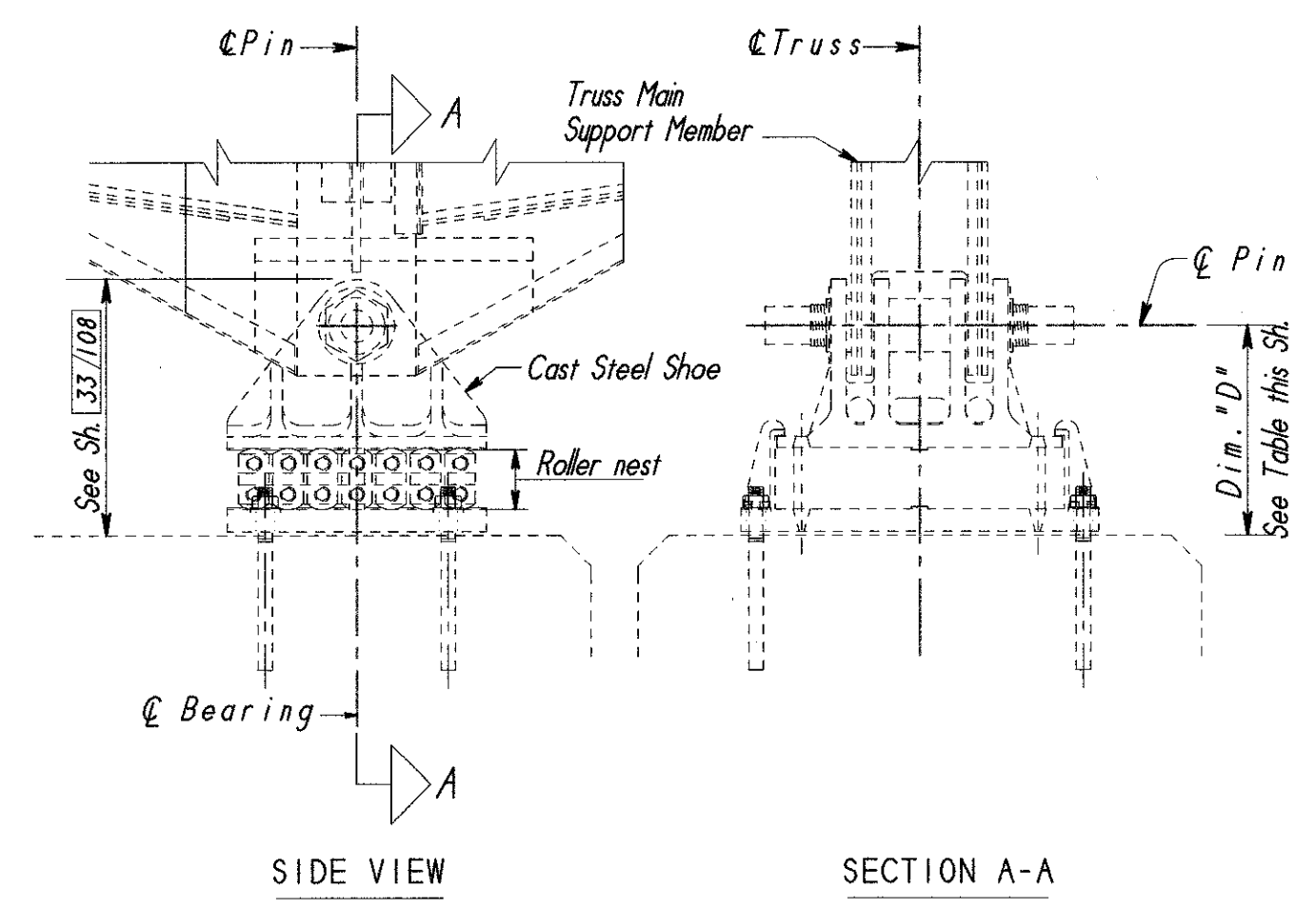
TYPICAL MASONRY PLATE

(See Table A this sheet for dimensions)

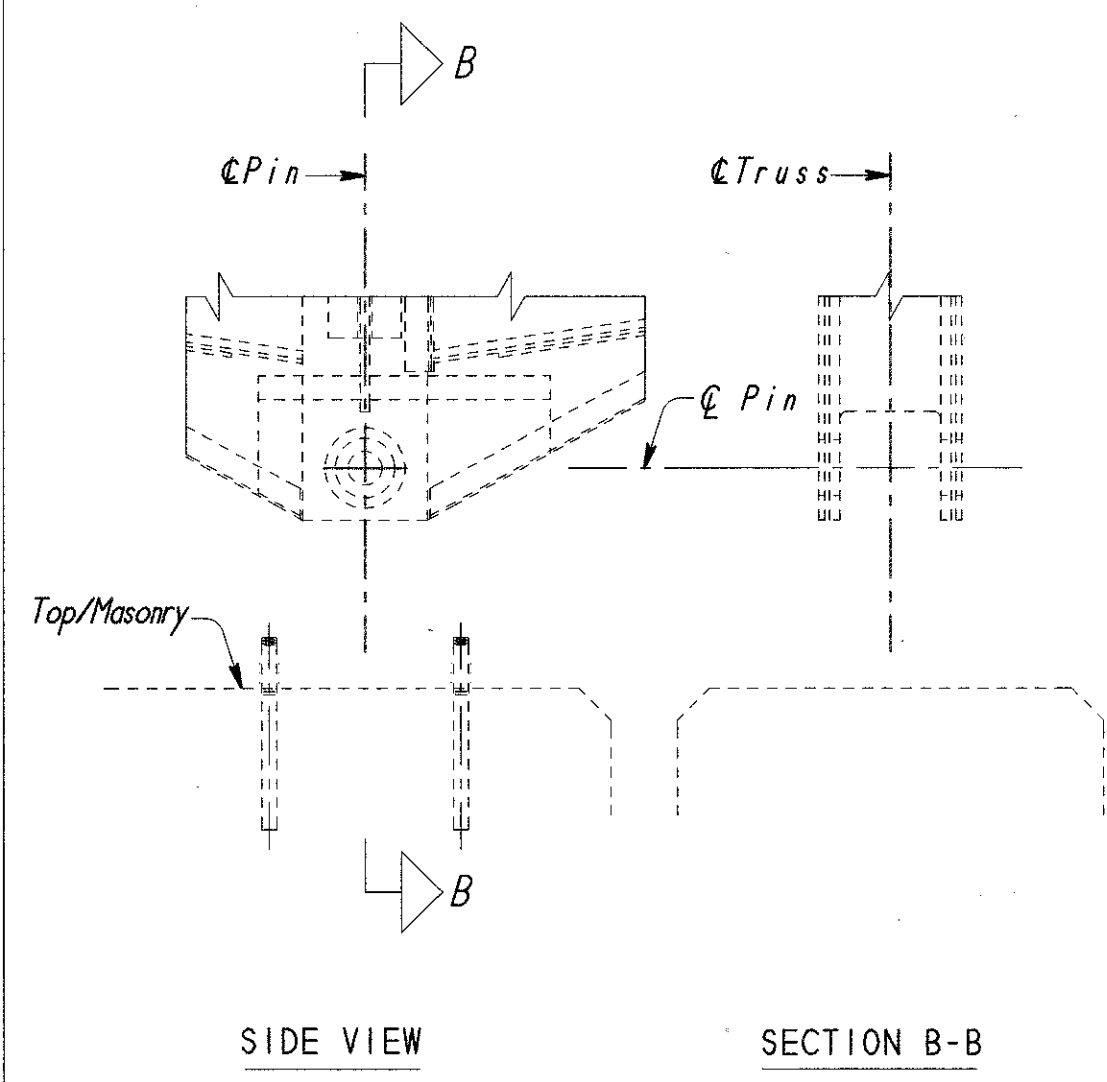
Pflum, Klausmeyer & Gehrm Consultants		36/108
TRUSS BEARING DEVICE PROPOSED POT BEARING		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

POTBRG Scale .6667

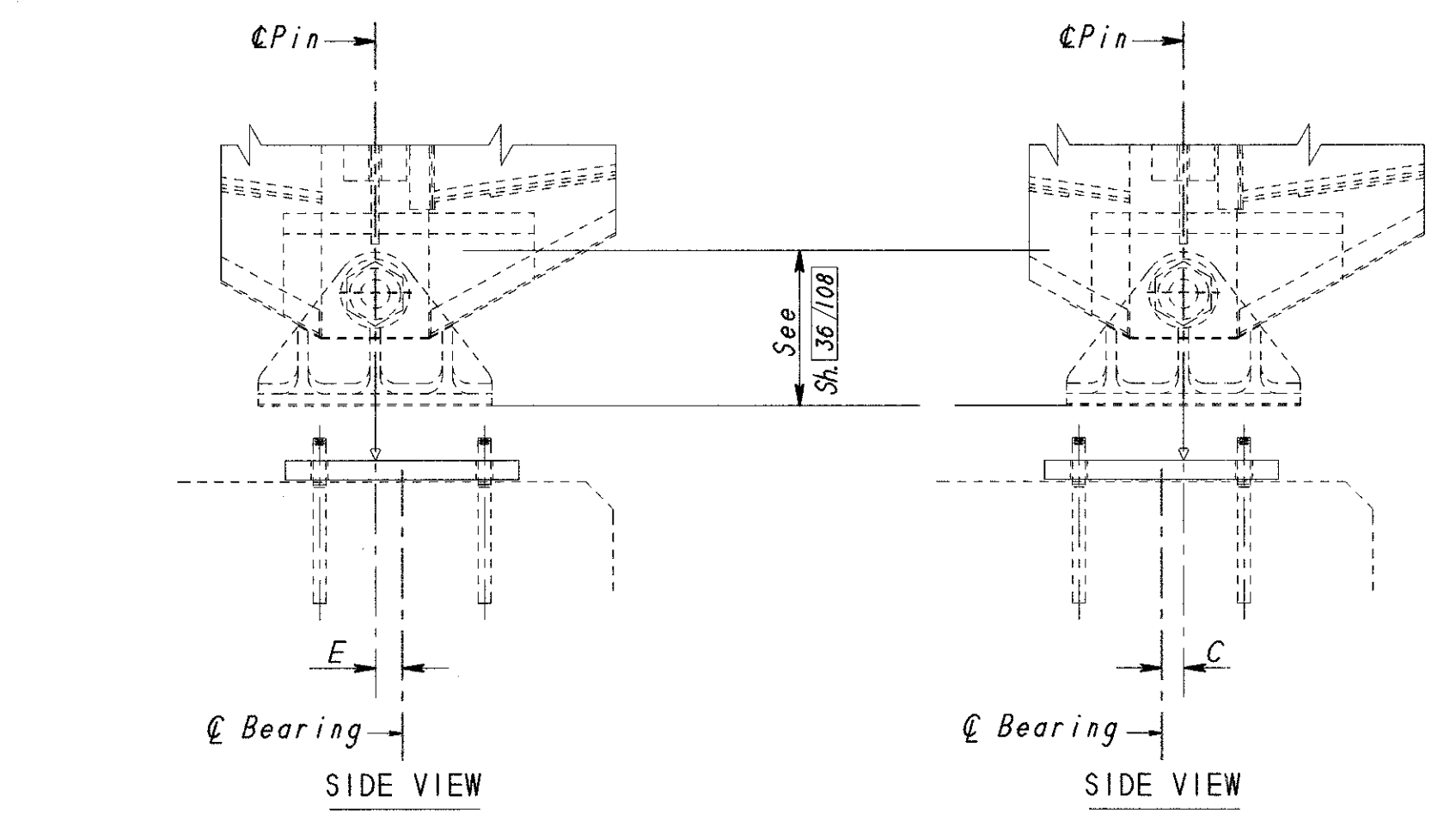
HAM-471-00.25



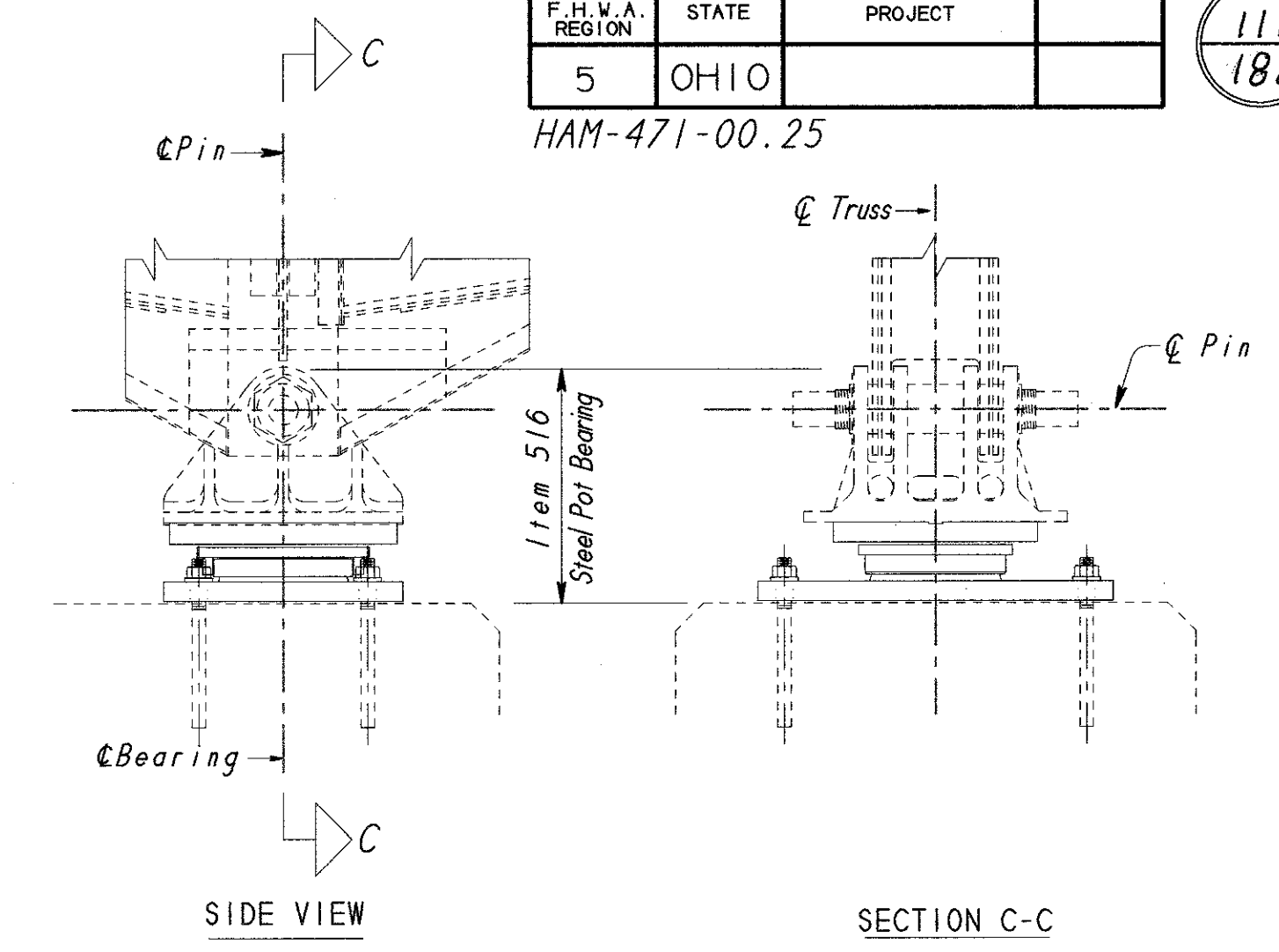
INITIAL CONDITIONS



CONDITIONS AFTER REMOVALS



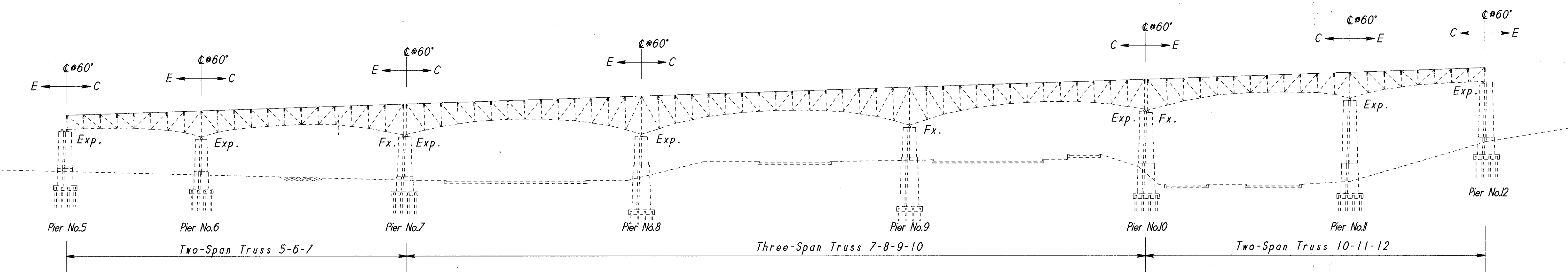
INTERMEDIATE CONDITIONS (PROPOSED)



FINAL CONDITIONS (PROPOSED)
(At 60°F Shown)

LEGEND:
E Denotes Expansion
C Denotes Contraction

LEGEND
Existing Structure
New Work



EXPANSION-CONTRACTION DIRECTIONAL KEY

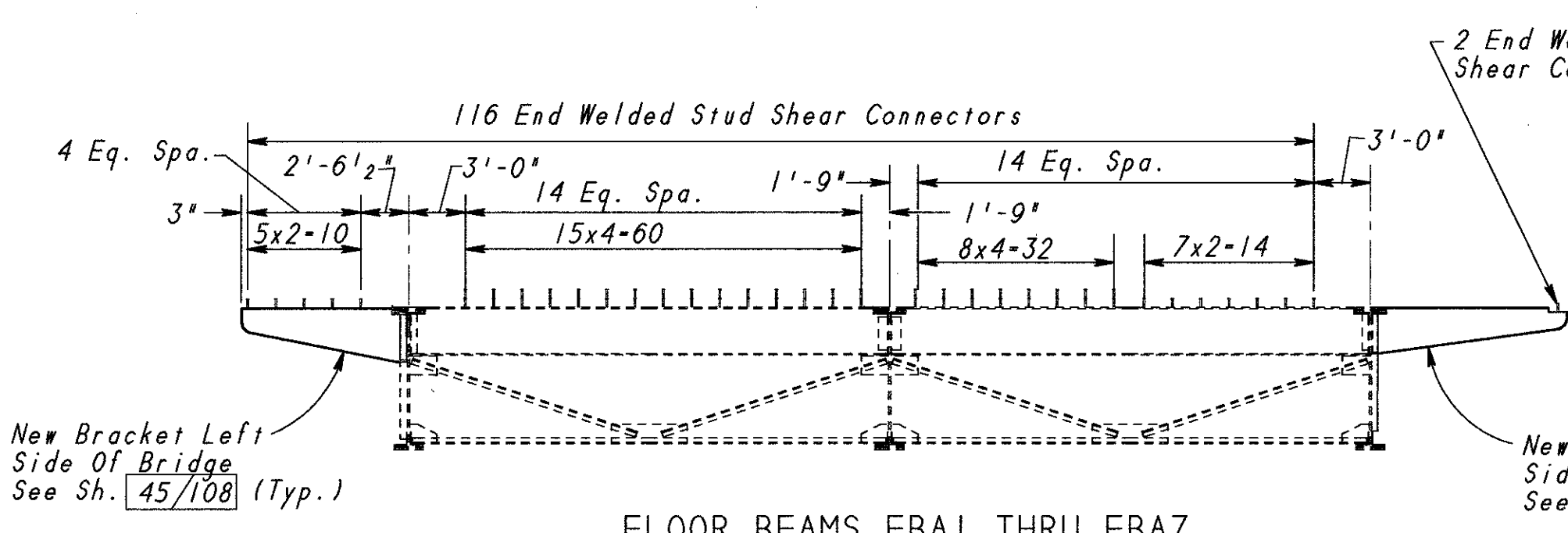
STRUCTURE	PIER	Guided Pot Bearing Type	Travel	AVERAGE TEMPERATURE OF STEEL STRUCTURE											
				10°	20°	30°	40°	50°	60°	70°	80°	90°	100°	110°	
CONTINUOUS TWO-SPAN TRUSS (SPANS 5-6-7)	No. 5	PB300E	C	27 ³² "	21 ³² "	1 ² "	11 ³² "	5 ³² "	0						
			E					0	5 ³² "	11 ³² "	1 ² "	21 ³² "	27 ³² "		
	No. 6	PBI000E	C	1 ² "	13 ³² "	5 ¹⁶ "	3 ¹⁶ "	3 ³² "	0						
			E					0	3 ³² "	3 ¹⁶ "	5 ¹⁶ "	13 ³² "	1 ² "		
CONTINUOUS THREE-SPAN TRUSS (SPANS 7-8-9-10)	No. 7	PB500E	C	11 ⁴ "	1"	3 ⁴ "	1 ² "	1 ⁴ "	0						
			E					0	1 ⁴ "	1 ² "	3 ⁴ "	1"	11 ⁴ "		
	No. 8	PBI200E	C	21 ³² "	17 ³² "	13 ³² "	1 ⁴ "	1 ⁸ "	0						
			E					0	1 ⁸ "	1 ⁴ "	13 ³² "	17 ³² "	21 ³² "		
No. 10	PB500E	C	19 ³² "	15 ³² "	11 ³² "	7 ³² "	1 ⁸ "	0							
		E					0	1 ⁸ "	7 ³² "	11 ³² "	15 ³² "	19 ³² "			
CONTINUOUS TWO-SPAN TRUSS (SPANS 10-11-12)	No. 11	PBI000E	C	1 ² "	13 ³² "	5 ¹⁶ "	3 ¹⁶ "	3 ³² "	0						
			E					0	3 ³² "	3 ¹⁶ "	5 ¹⁶ "	13 ³² "	1 ² "		
	No. 12	PB300E	C	27 ³² "	21 ³² "	1 ² "	11 ³² "	5 ³² "	0						
			E					0	5 ³² "	11 ³² "	1 ² "	21 ³² "	27 ³² "		

DESCRIPTION	EXISTING PIN HEIGHT DIM. 'D'						
	PIER NO.						
	5	6	7	8	10	11	12
Continuous Two-Span Truss (Spans 5-6-7)	1'-9 ¹ / ₈ " ±	2'-3 ¹ / ₈ " ±					
Continuous Three-Span Truss (Spans 7-8-9-10)			1'-9 ¹ / ₈ " ±	2'-5 ¹ / ₈ " ±	1'-9 ¹ / ₈ " ±		
Continuous Two-Span Truss (Spans 10-11-12)					2'-3 ¹ / ₈ " ±	1'-9 ¹ / ₈ " ±	

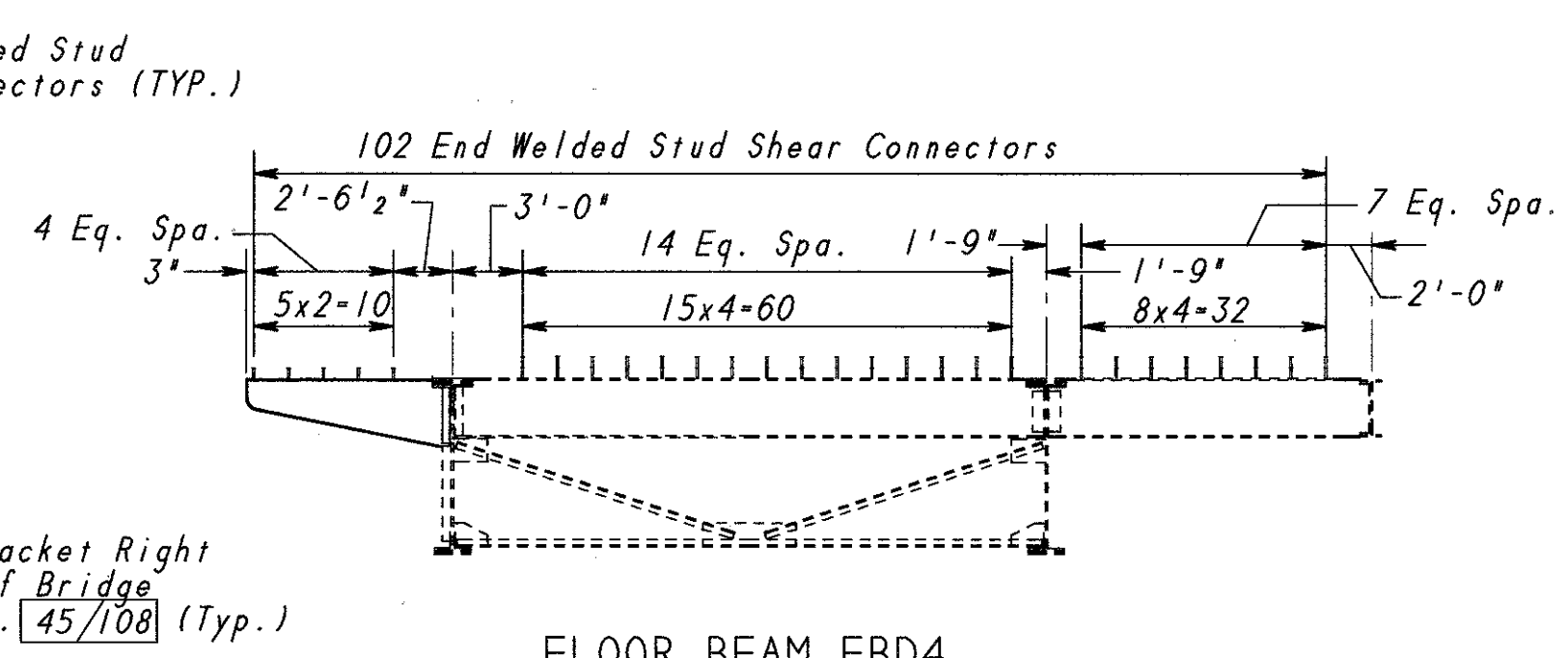
- NOTES:
1. Work this sheet with Sheets 30/108 thru 32/108, and Sh. 36/108.
 2. See GENERAL NOTES, Sh. 7/108 Thru Sh. 11/108.
 3. Representation of existing structure is approximate.

Pflum, Klausmeier & Gehrum Consultants		37/108
TRUSS BEARING DEVICES POT BEARINGS SETTING SCHEDULE BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/PLF	WDD
CHECKED	REVIEWED	DATE
ED/PLF	IEH	April 96

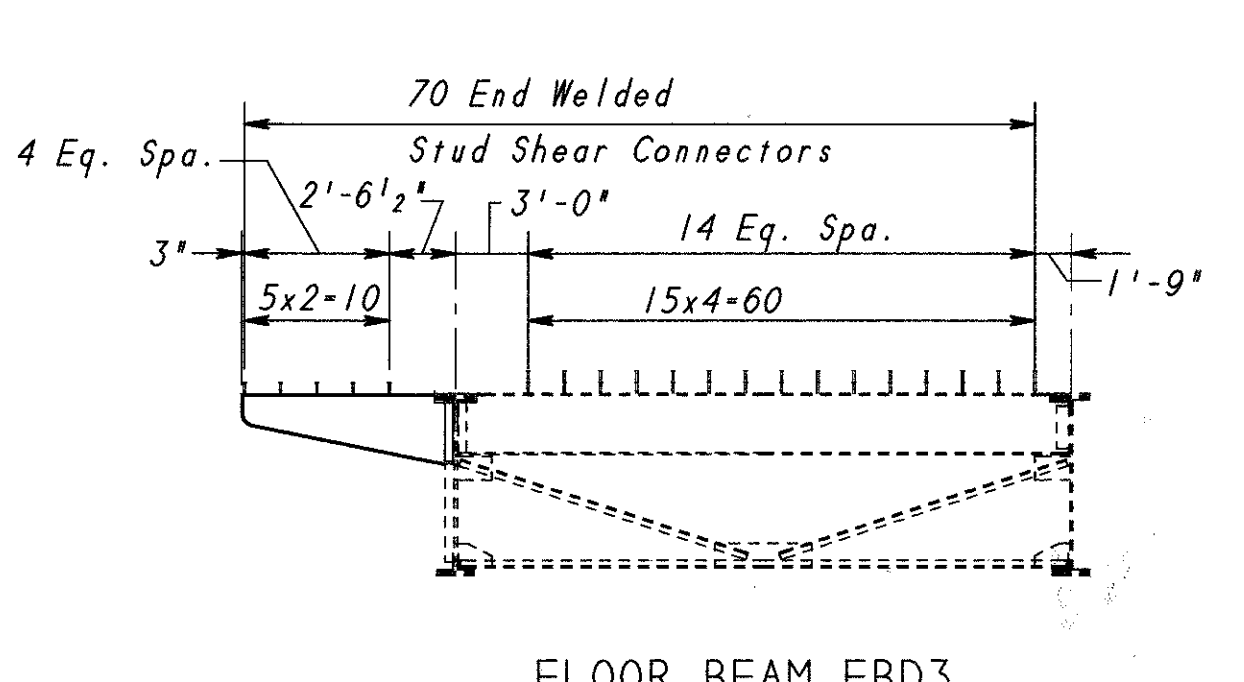
BRIDGE No. 2



FLOOR BEAMS FBA1 THRU FBA7
FLOOR BEAM FBB1
FLOOR BEAMS FBD5 THRU FBD9*
(* No Bracket On Right Side Of FBD5 And Left Side Of FBD9)

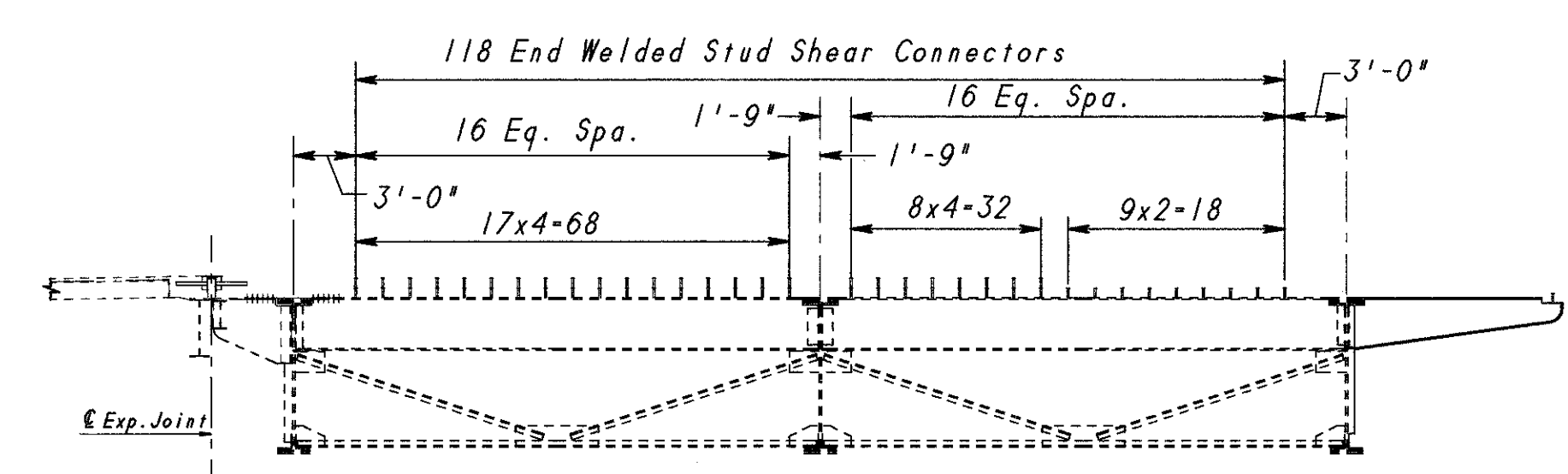


FLOOR BEAM FBD4

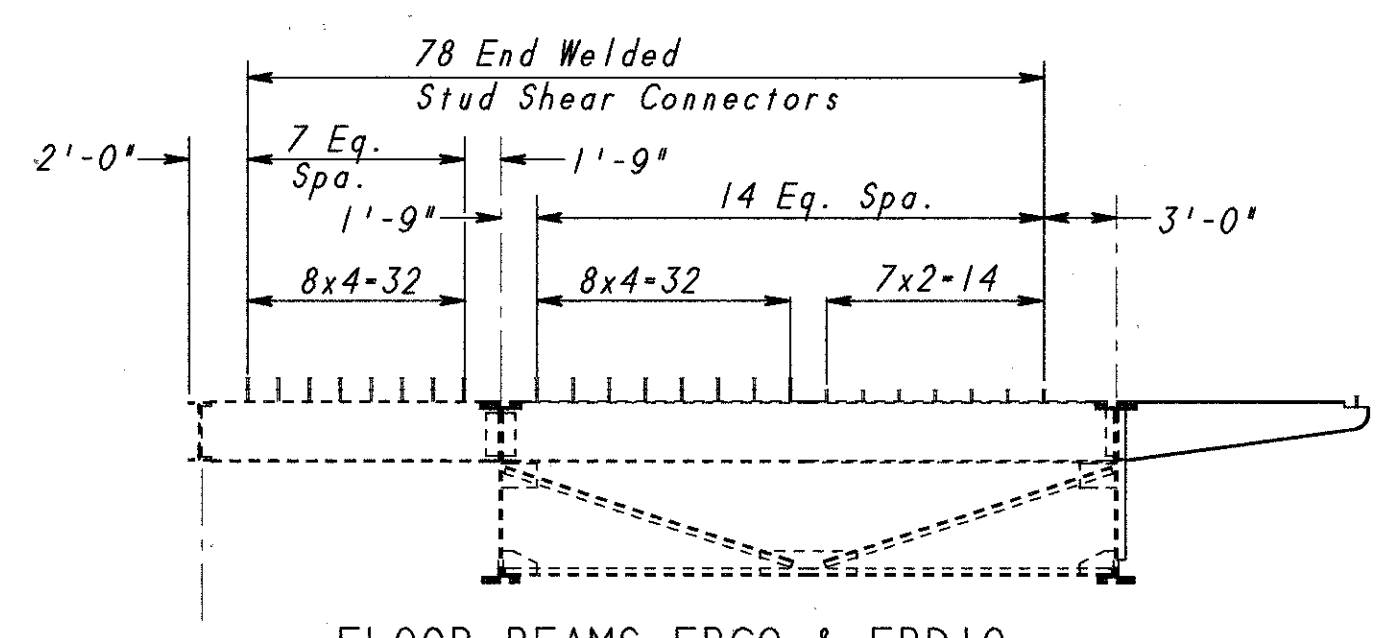


FLOOR BEAM FBD3

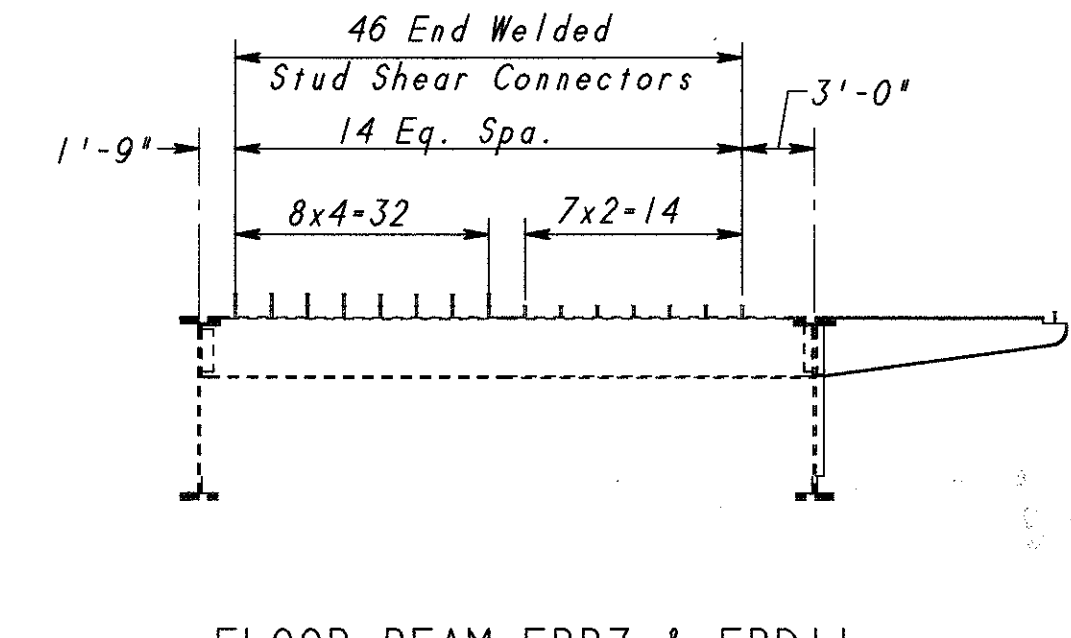
- NOTES:
1. - Work this sheet with Sh. 41/108 & 45/108
 2. - All views shown are looking Forward.
 3. - See General Notes Sh. 7/108 Thru Sh. 11/108.



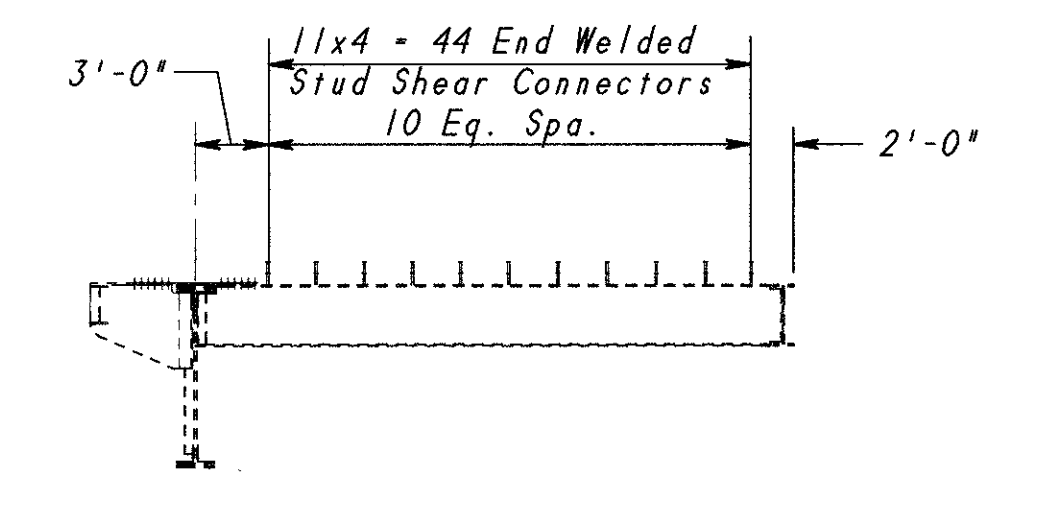
FLOOR BEAMS FBB2 THRU FBB6 & FBB8



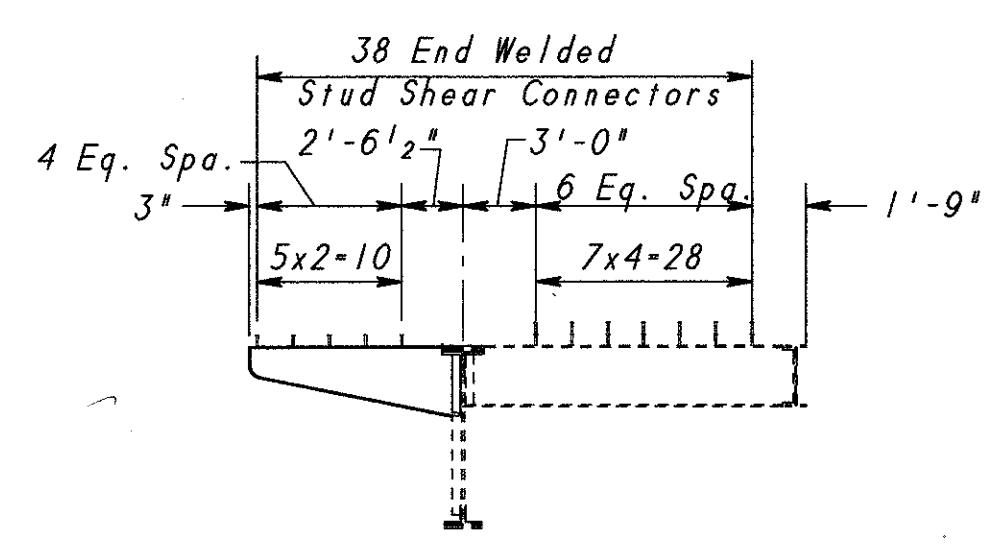
FLOOR BEAMS FBC9 & FBD10



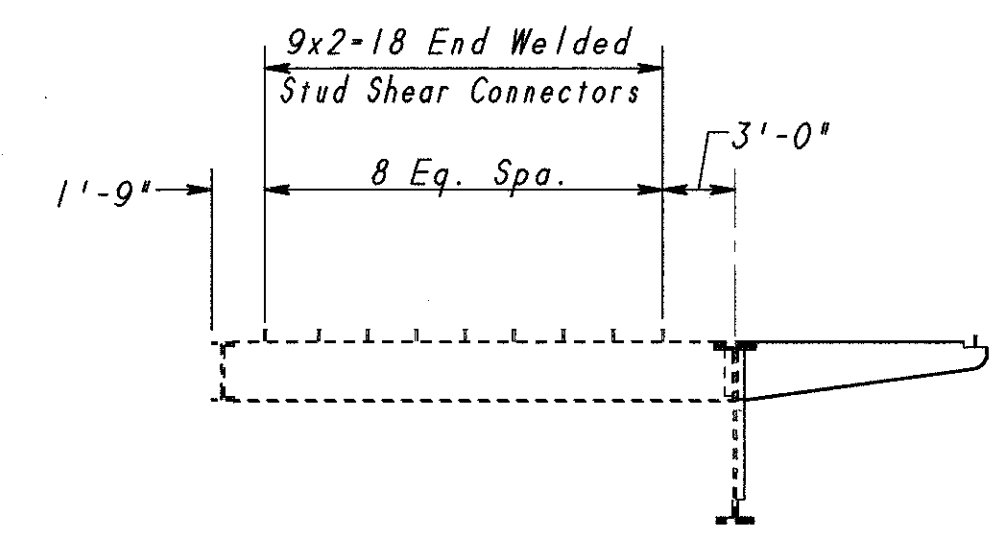
FLOOR BEAM FBB7 & FBD11



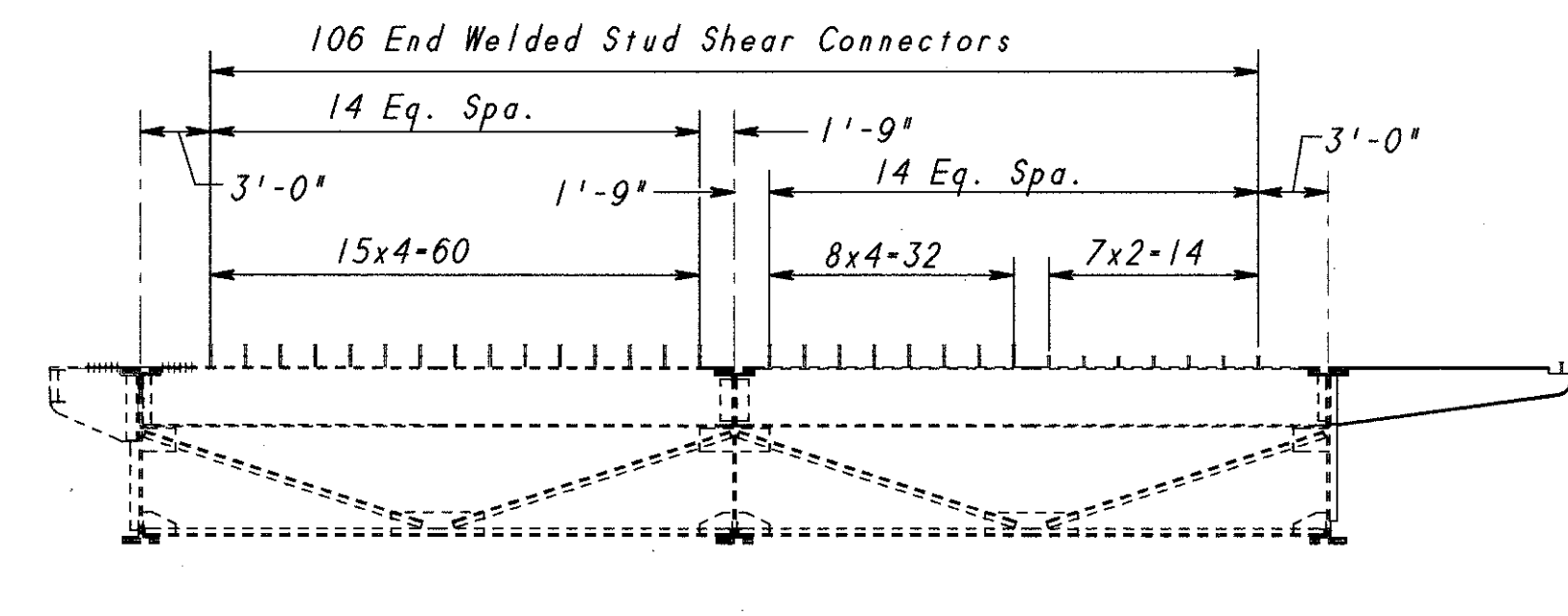
FLOOR BEAM FBC2



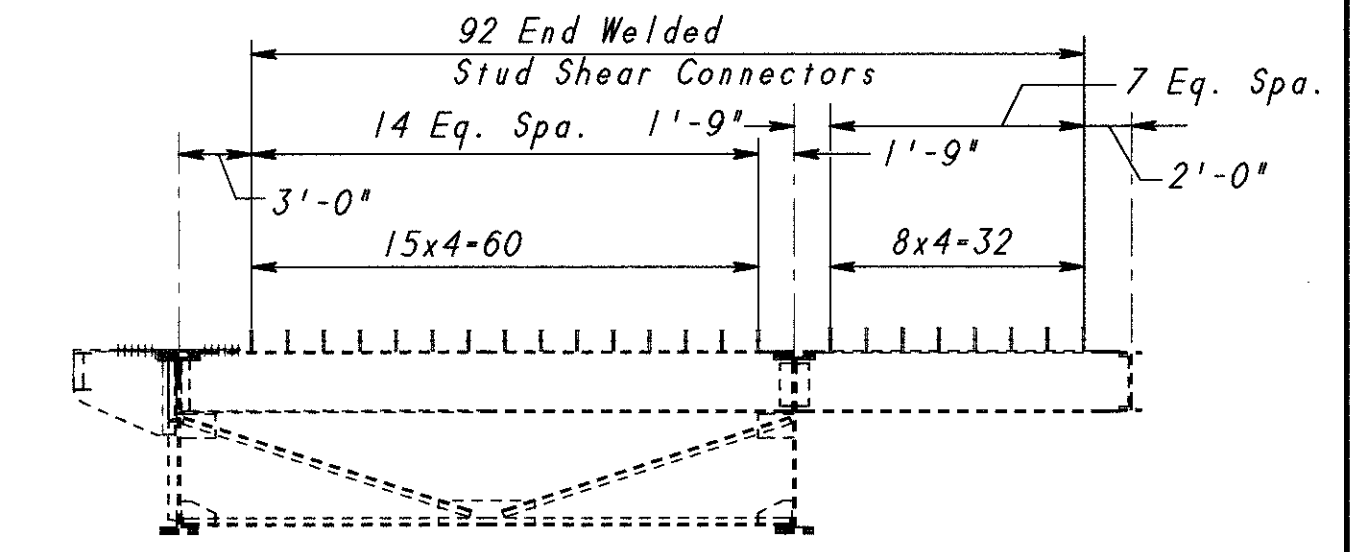
FLOOR BEAMS FBD2



FLOOR BEAMS FBC10 & FBD12

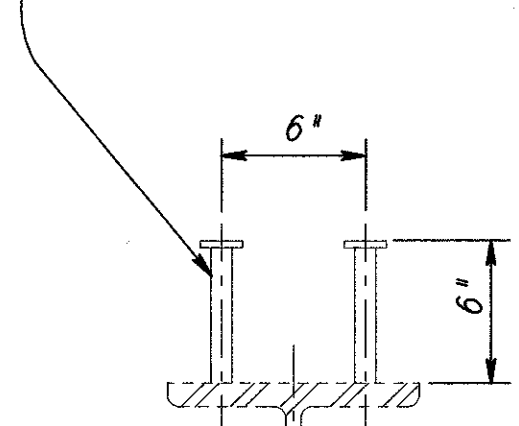


FLOOR BEAMS FBC4 THRU FBC8

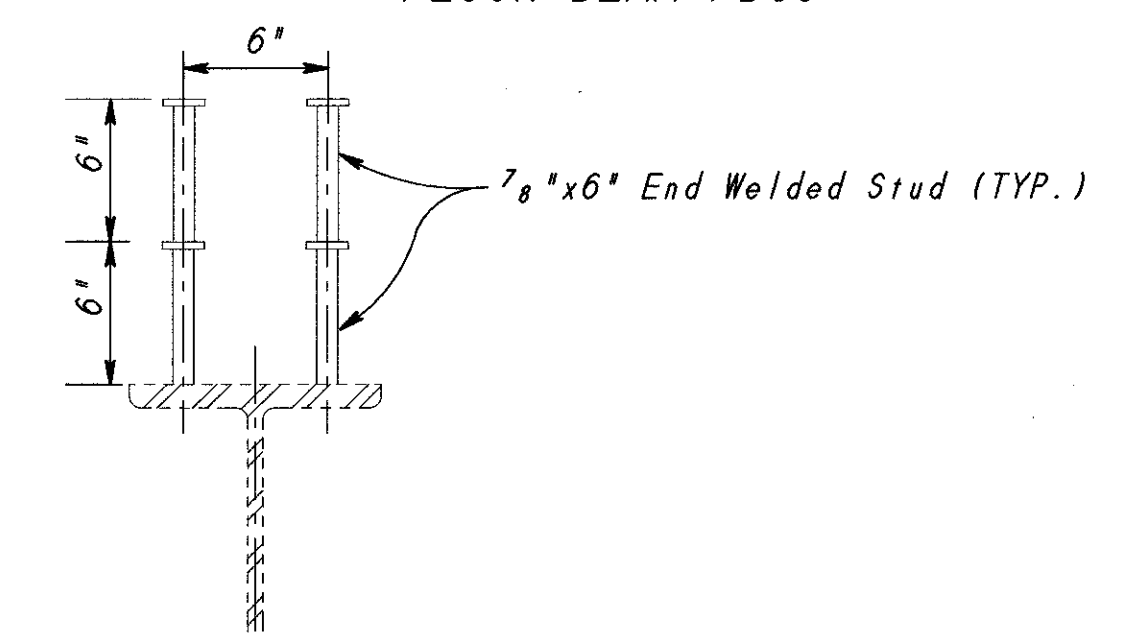


FLOOR BEAM FBC3

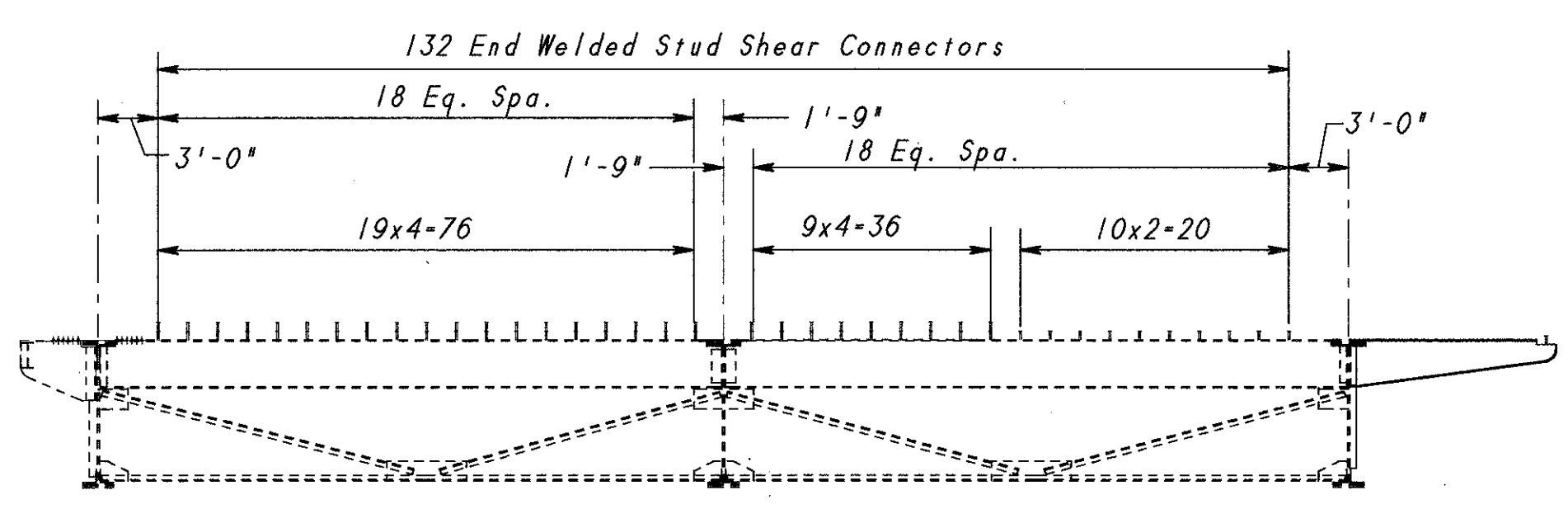
7/8"x6" End Welded Stud Shear Connectors ASTM A108 (Typical)



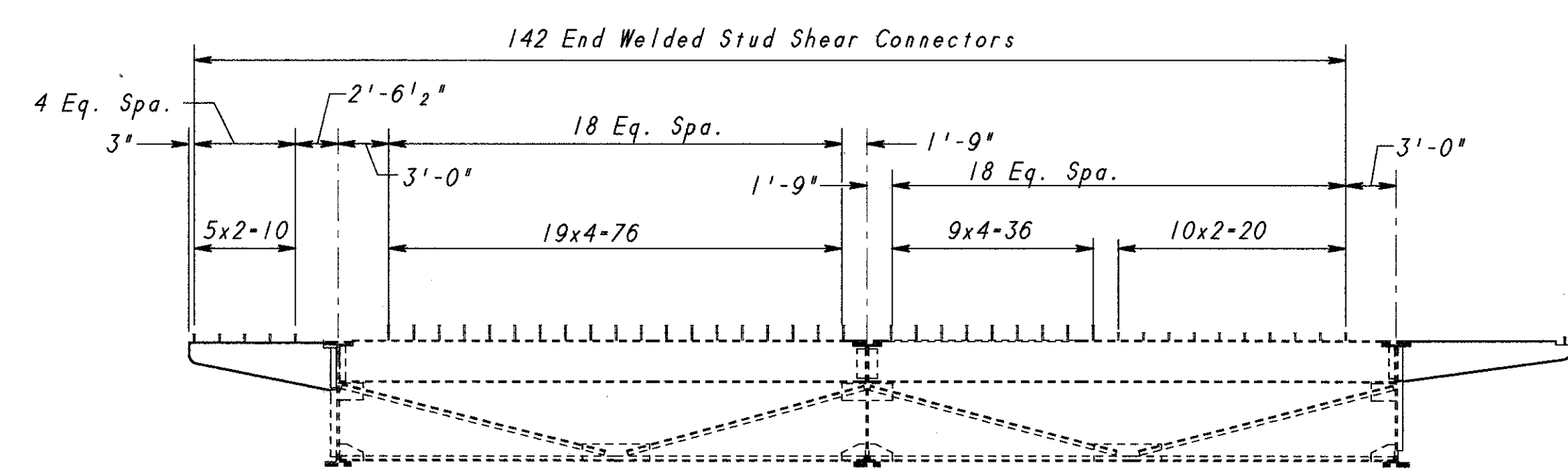
2 STUDS TYPICAL SECTION



4 STUDS TYPICAL SECTION



FLOOR BEAMS FBC1 & FBC11



FLOOR BEAM FBD1 & FBD13

LEGEND

Existing Structure

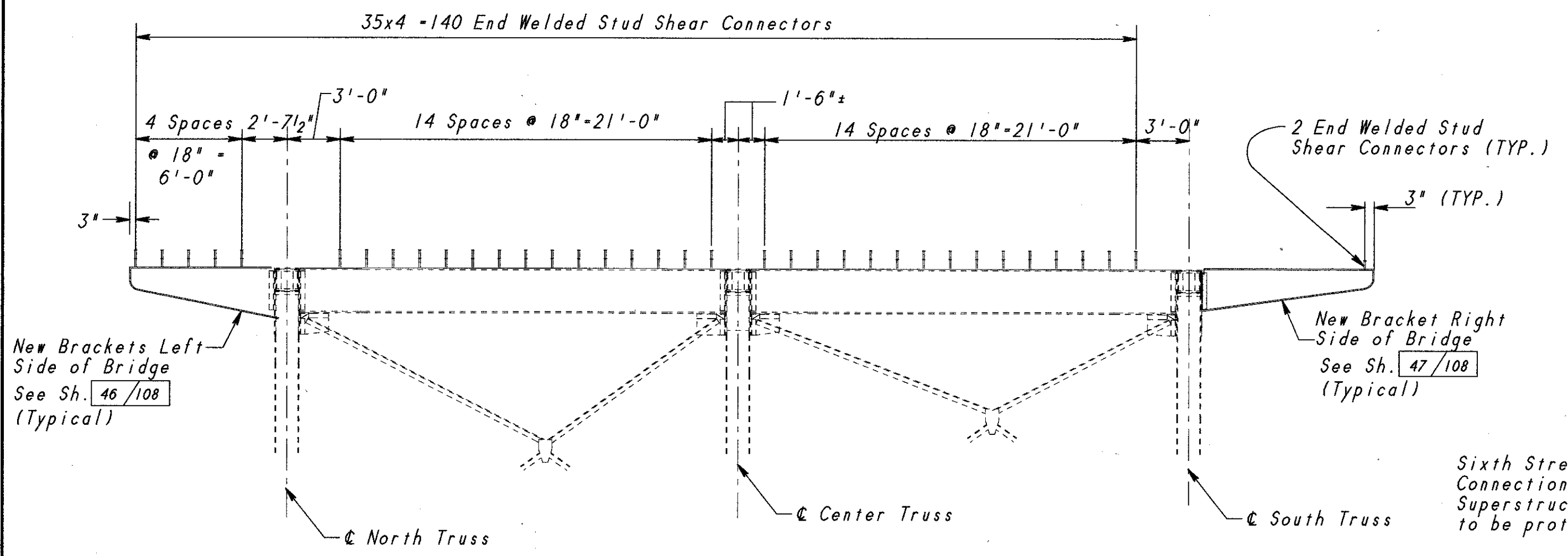
New Work

FBA1 Denotes Floor Beam Unit A Number 1

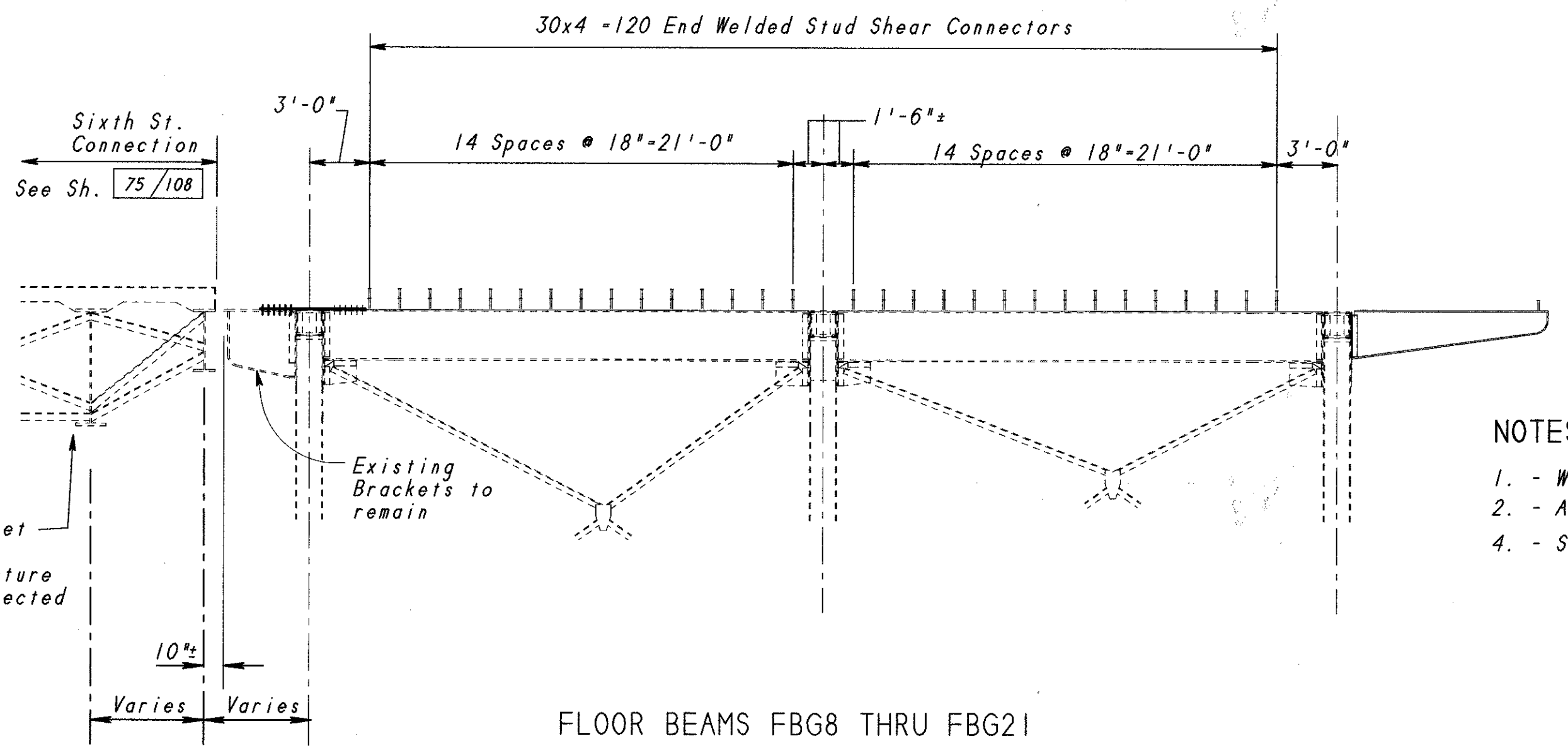
Pflum, Klausmeyer & Gehrum		38 / 108
CONSULTANTS		OHIO
STRUCTURAL STEEL DETAILS		
STUD SHEAR CONNECTORS		
SPANS 1-2 THRU 4-5		
(UNITS A THRU D)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
DESIGNED		Sta. 47+16.19
ED	JDR	
TRACED	WDD	
CHECKED	IEH	
REVIEWED	DATE	REVISION
	April 96	

STUDET Scale 1/6

STUD SHEAR CONNECTORS CMS 513.171
TOTAL THIS SHEET - 3964

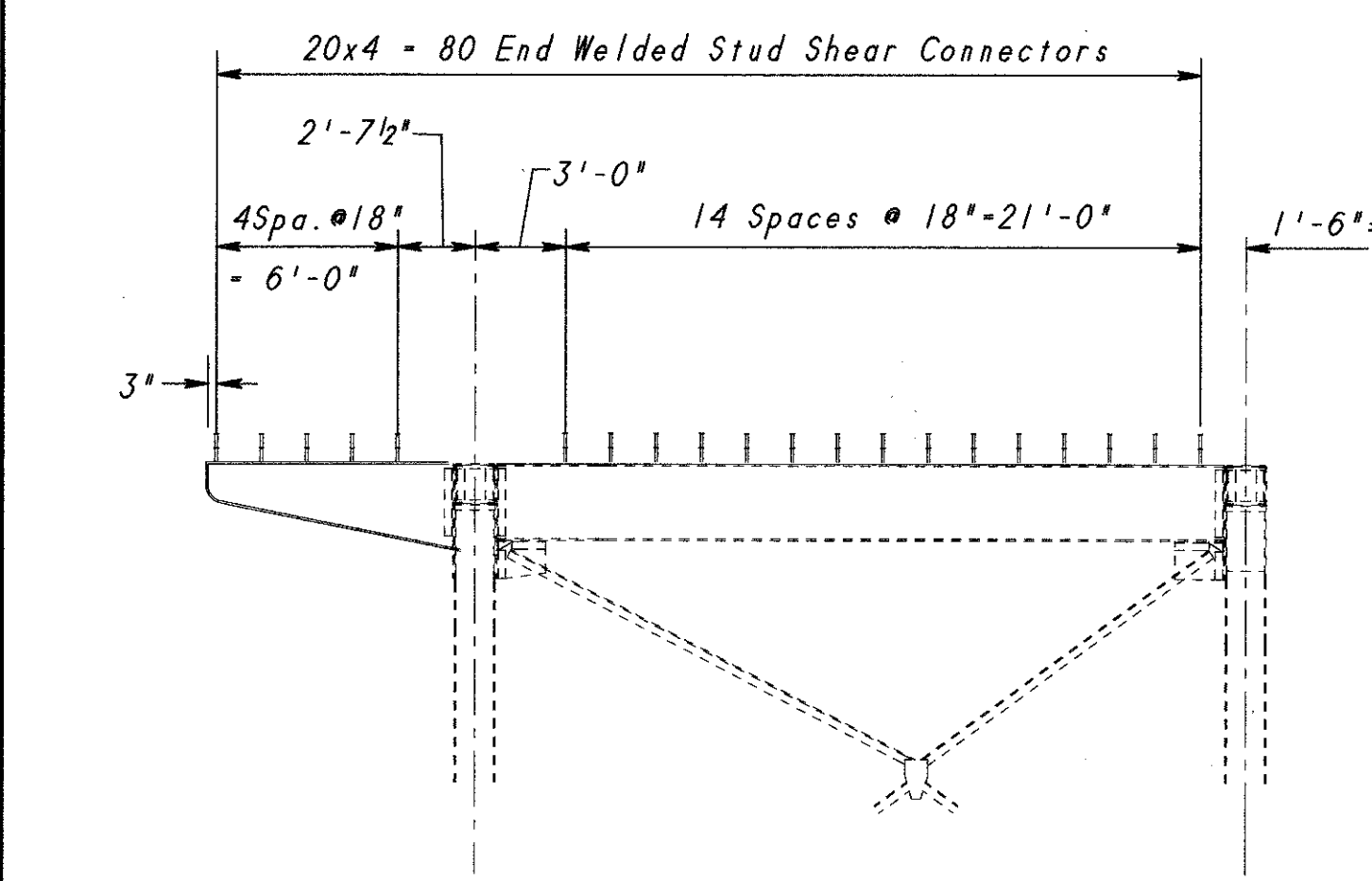


FLOOR BEAMS FBE5 THRU FBE21
FLOOR BEAMS FBF5 THRU FBF45
FLOOR BEAMS FBG5 THRU FBG7

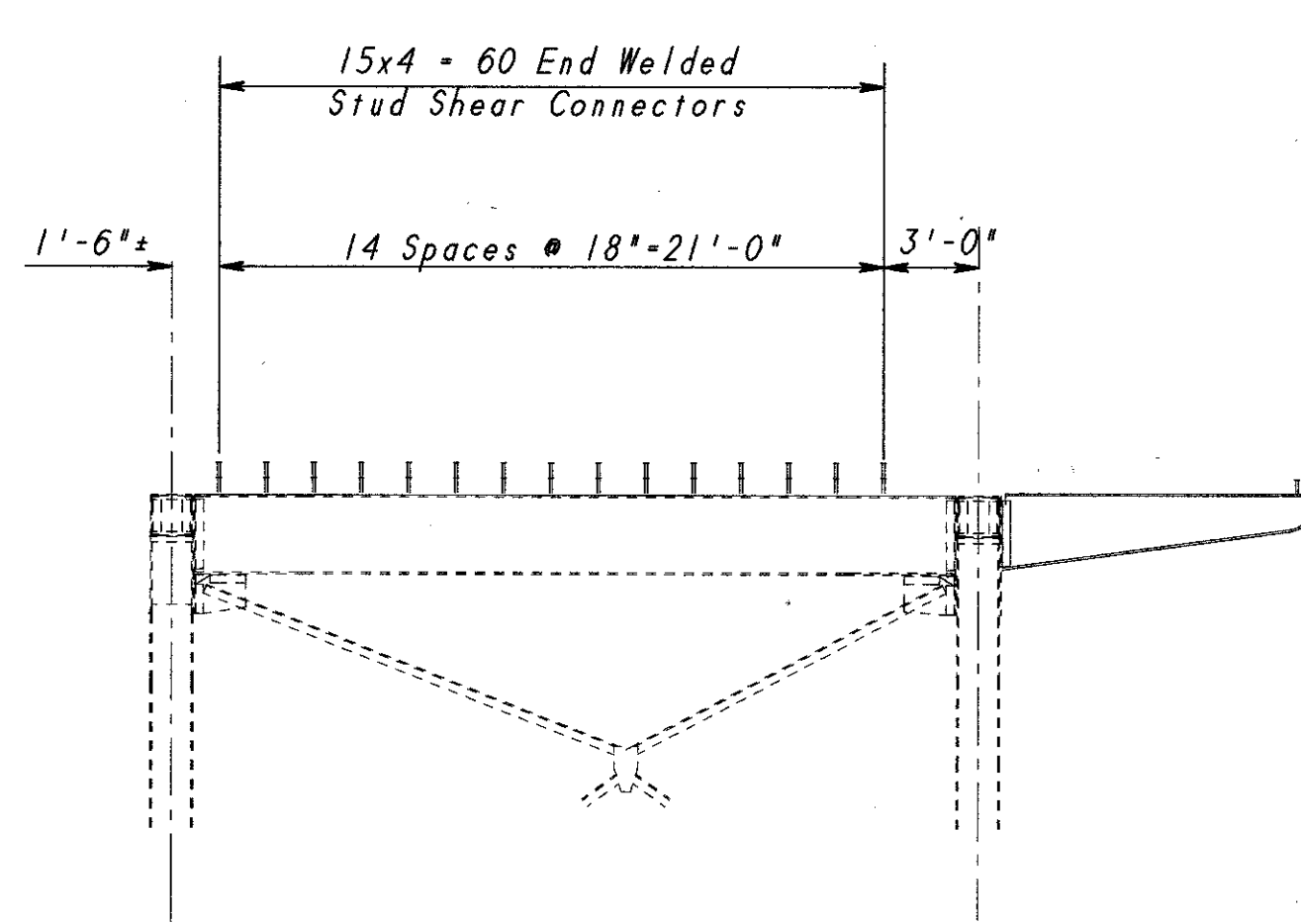


FLOOR BEAMS FBG8 THRU FBG21

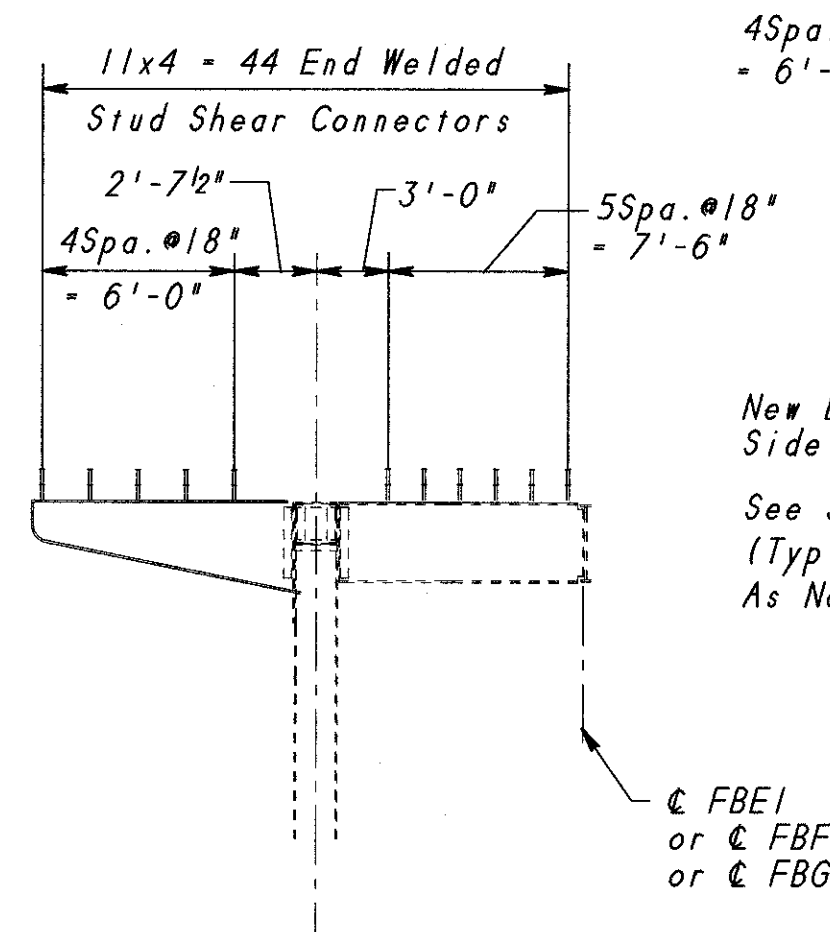
- NOTES:
1. - Work this sheet with Sh. 42/108, 46/108 & 47/108.
 2. - All views shown are looking Forward.
 4. - See General Notes Sh. 7/108 Thru 11/108.



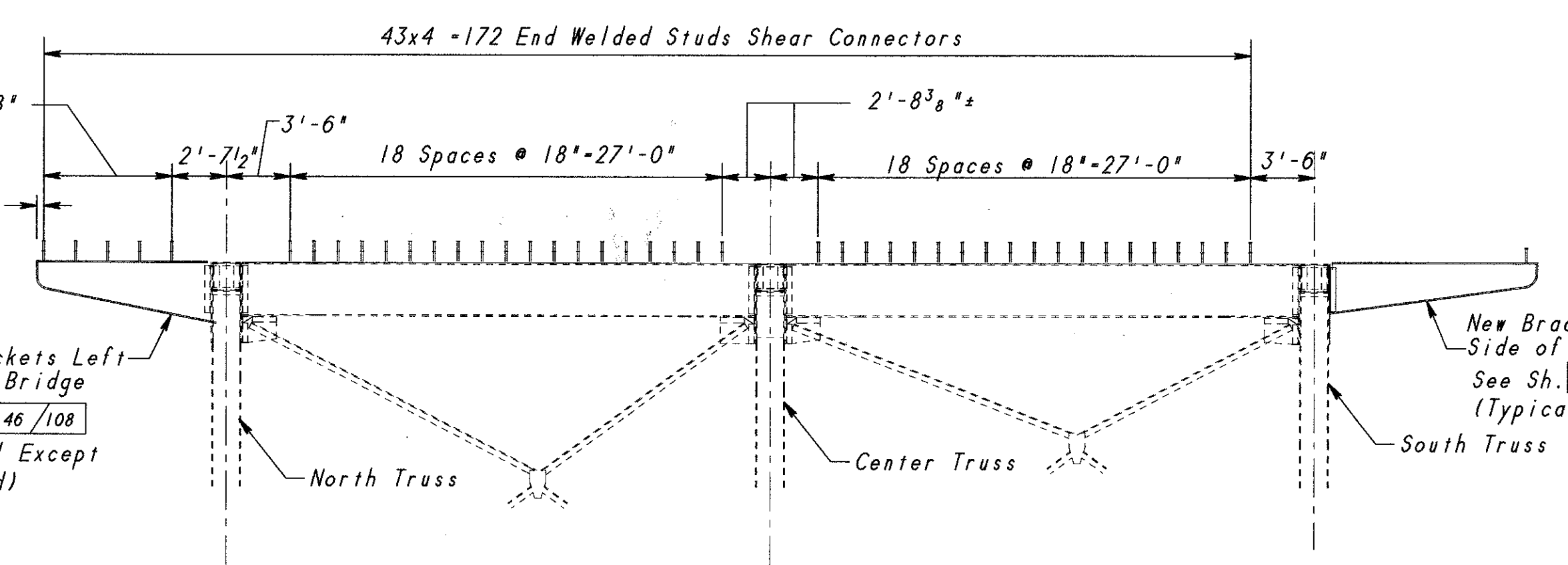
FLOOR BEAM FBE3
FLOOR BEAM FBF3
FLOOR BEAM FBG3



FLOOR BEAM FBE23
FLOOR BEAM FBF47
FLOOR BEAM FBG23

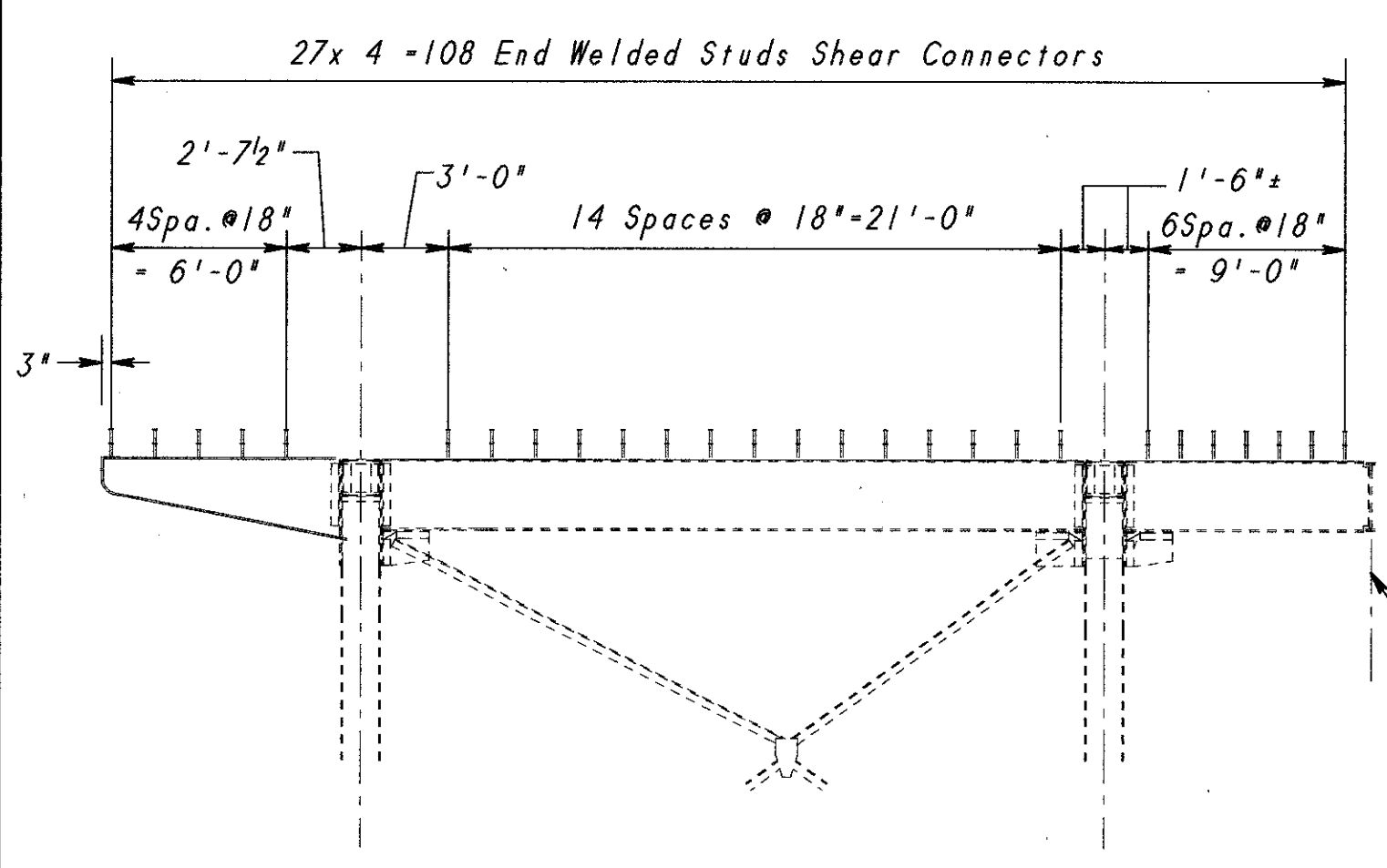


FLOOR BEAM FBE2
FLOOR BEAM FBF2
FLOOR BEAM FBG2

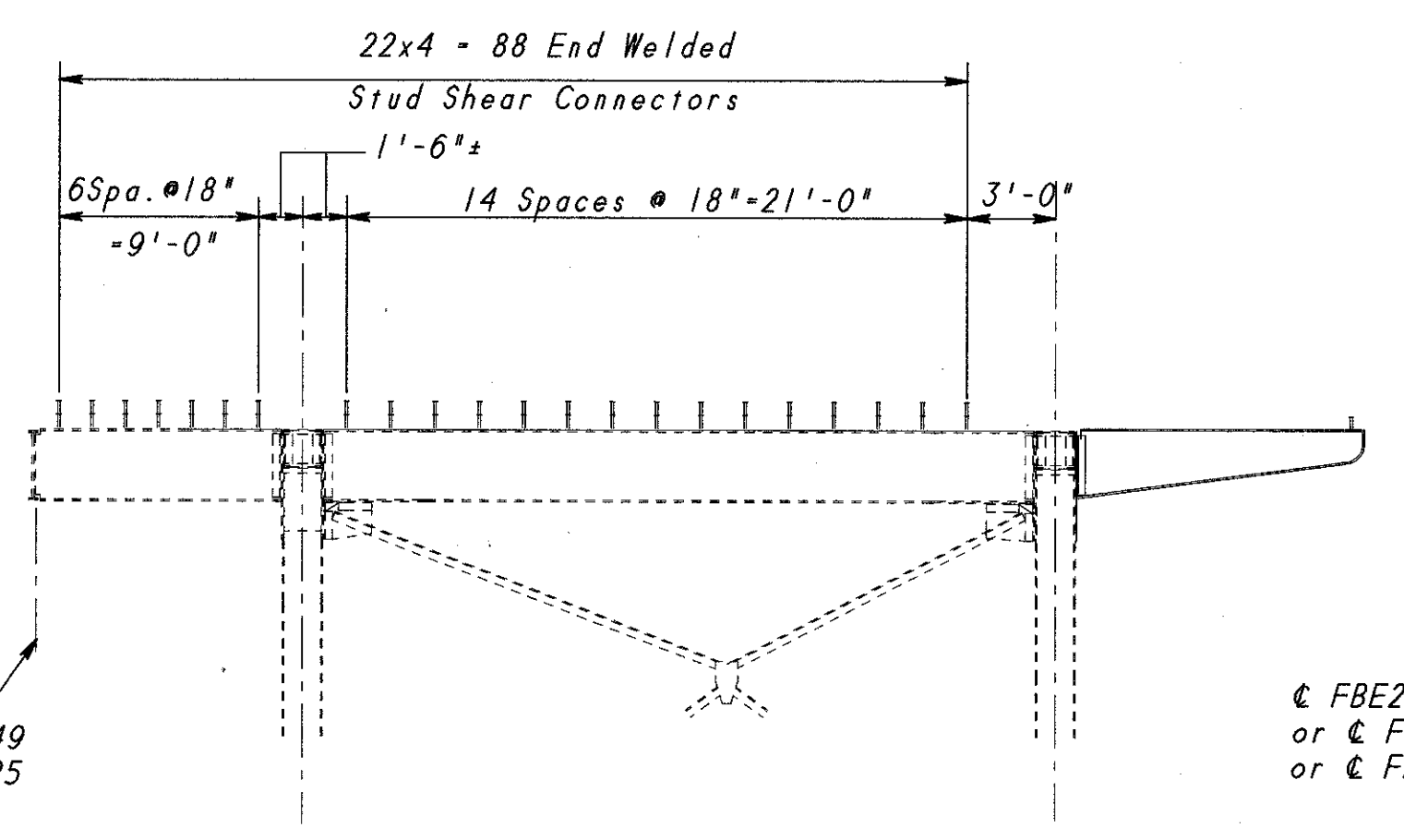


FLOOR BEAMS FBE1 & FBE25
FLOOR BEAMS FBF1 & FBF49
FLOOR BEAMS FBG1 & FBG25*

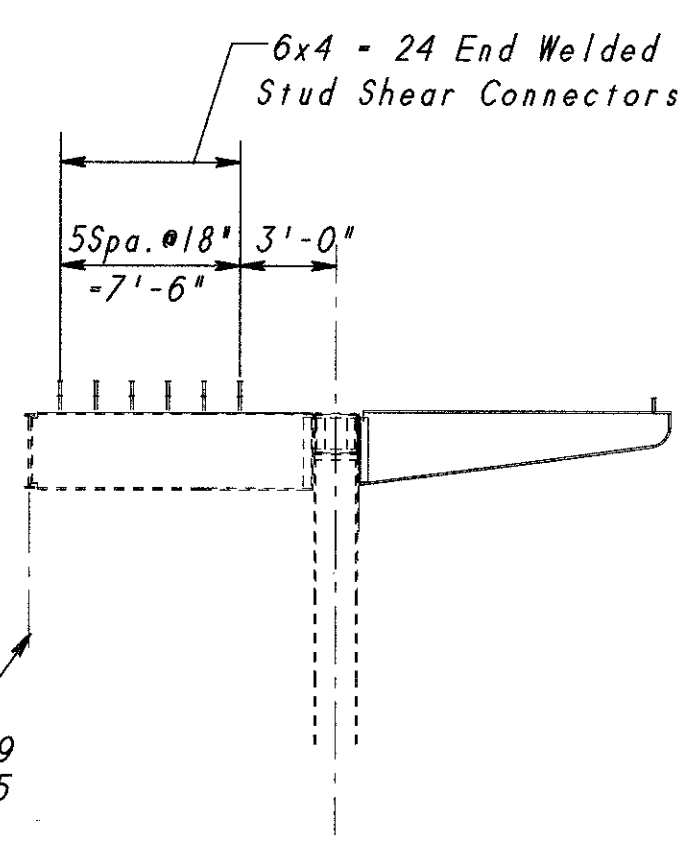
* No New Bracket On Left Side of FBG25



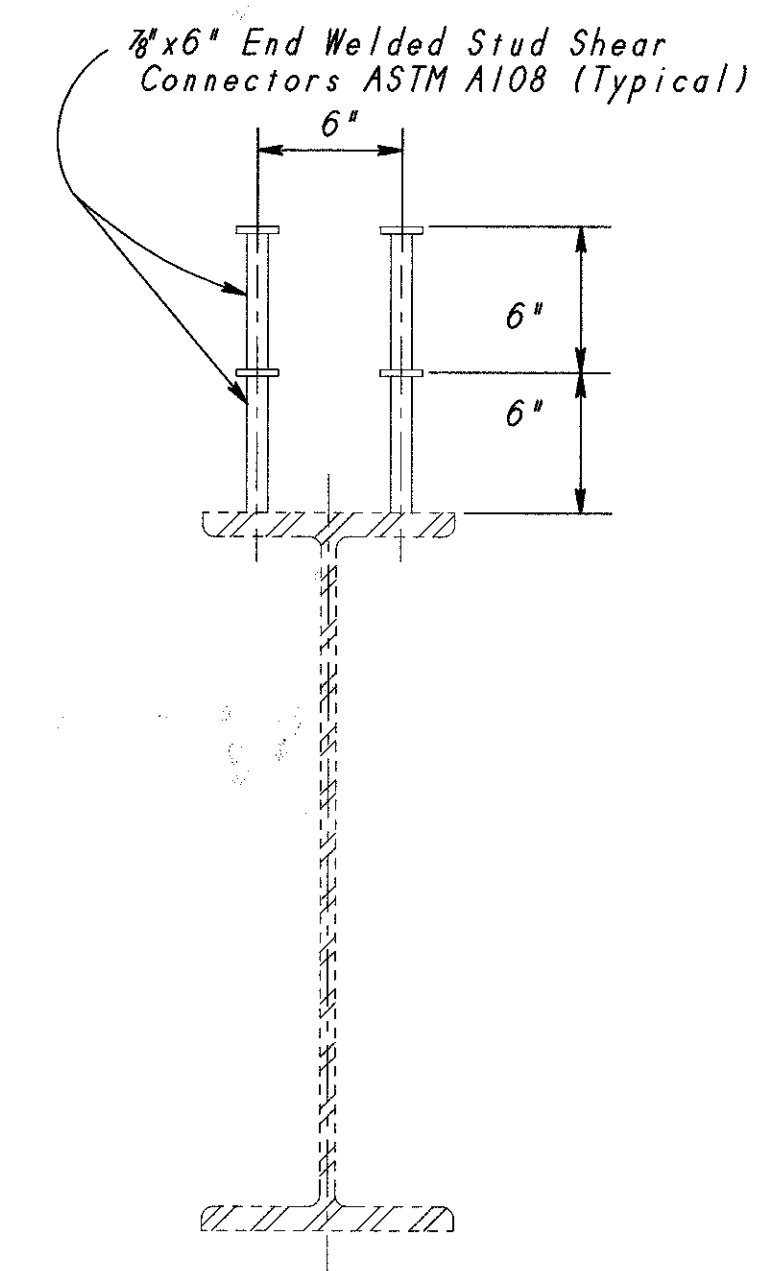
FLOOR BEAM FBE4
FLOOR BEAM FBF4
FLOOR BEAM FBG4



FLOOR BEAM FBE22
FLOOR BEAM FBF46
FLOOR BEAM FBG22



FLOOR BEAM FBE24
FLOOR BEAM FBF48
FLOOR BEAM FBG24

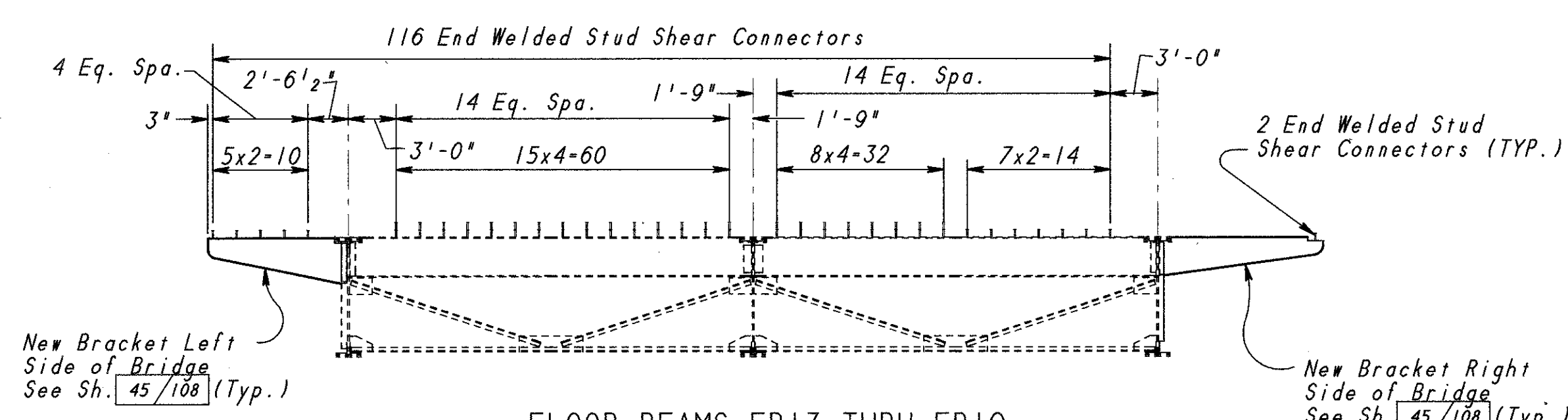


4 STUDS TYPICAL SECTION
STUD SHEAR CONNECTORS CMS 513.171
TOTAL THIS SHEET=12644

- LEGEND
- Existing Structure
 - New Work
 - FBE5 Denotes Floor Beam Unit E Number 5

Pflum, Klausmeyer & Gehrum Consultants		39/108
STRUCTURAL STEEL DETAILS STUD SHEAR CONNECTOR SPANS 5-6-7, 7-8-9-10 & 10-11-12 (UNITS E, F, & G) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY	DESIGNED	ED
ED/JDR	DRAWN	ED/JDR
WDD	TRACED	WDD
IEH April 96	CHECKED	IEH April 96
DATE	REVIEWED	DATE
Sta. 30+55.40	Sta. 47+16.19	REVISED

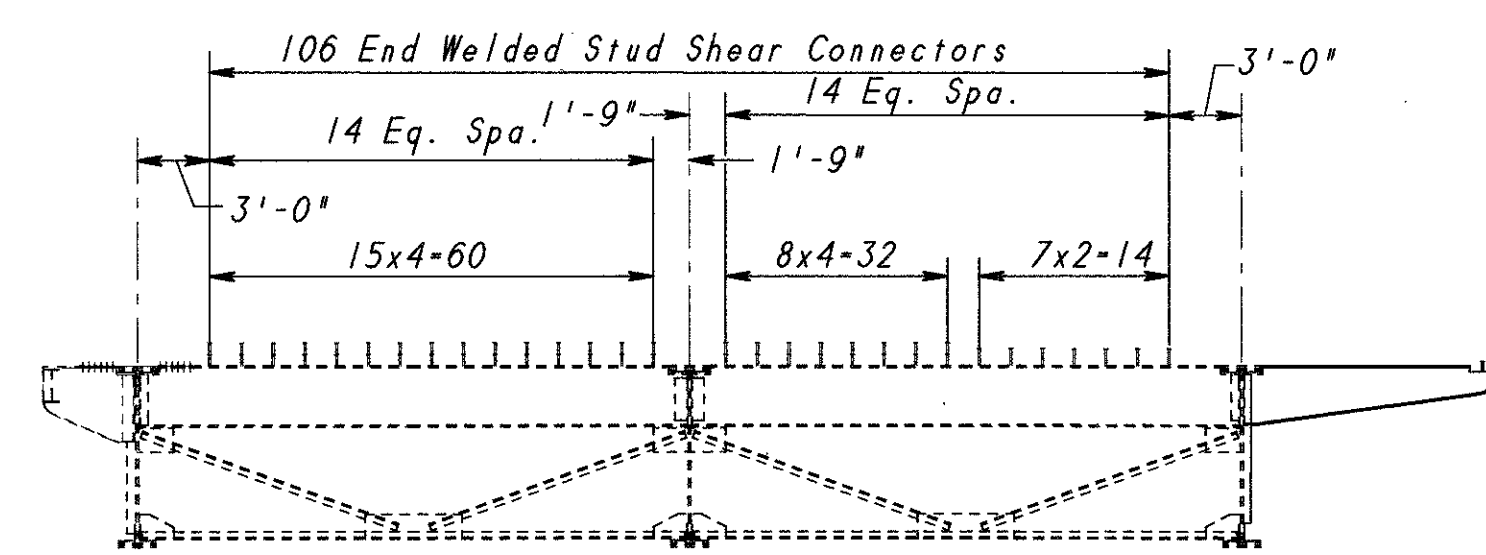
STUDS 1/8



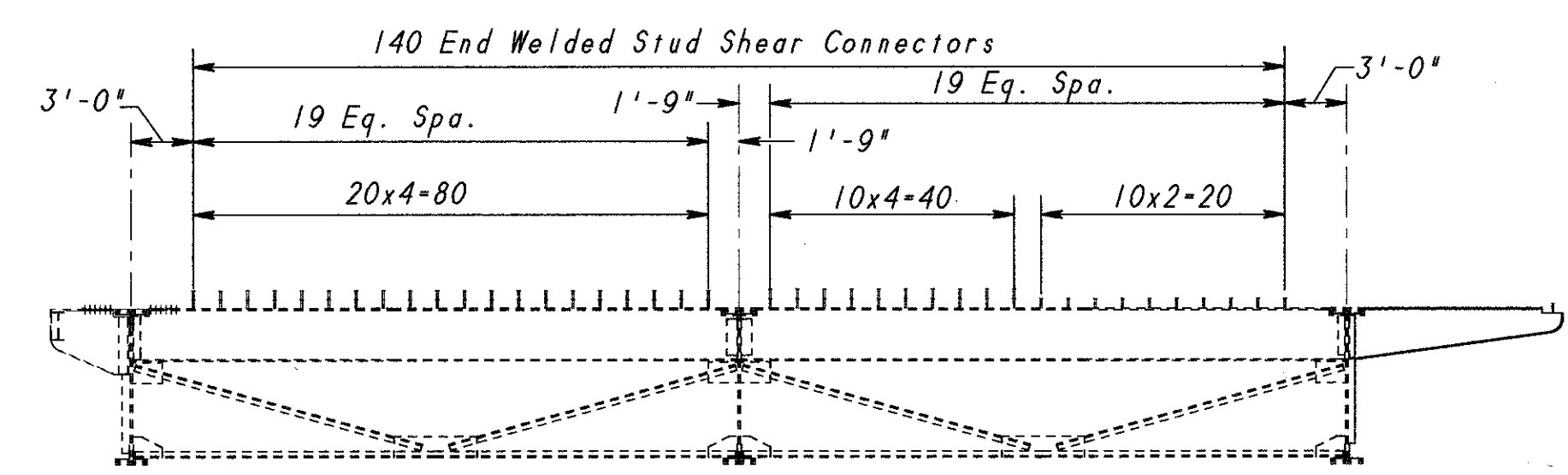
New Bracket Left Side of Bridge See Sh. 45/108 (Typ.)

New Bracket Right Side of Bridge See Sh. 45/108 (Typ.)

FLOOR BEAMS FB13 THRU FB19
FLOOR BEAMS FB11 THRU FB19
FLOOR BEAMS FBK1 THRU FBK7
FLOOR BEAMS FBL1 THRU FBL7
FLOOR BEAMS FBM1 THRU FBM7



FLOOR BEAMS FBH5 THRU FBH9*
FLOOR BEAMS FB11 THRU FB12
(* No Bracket On Right Side Of FBH5)

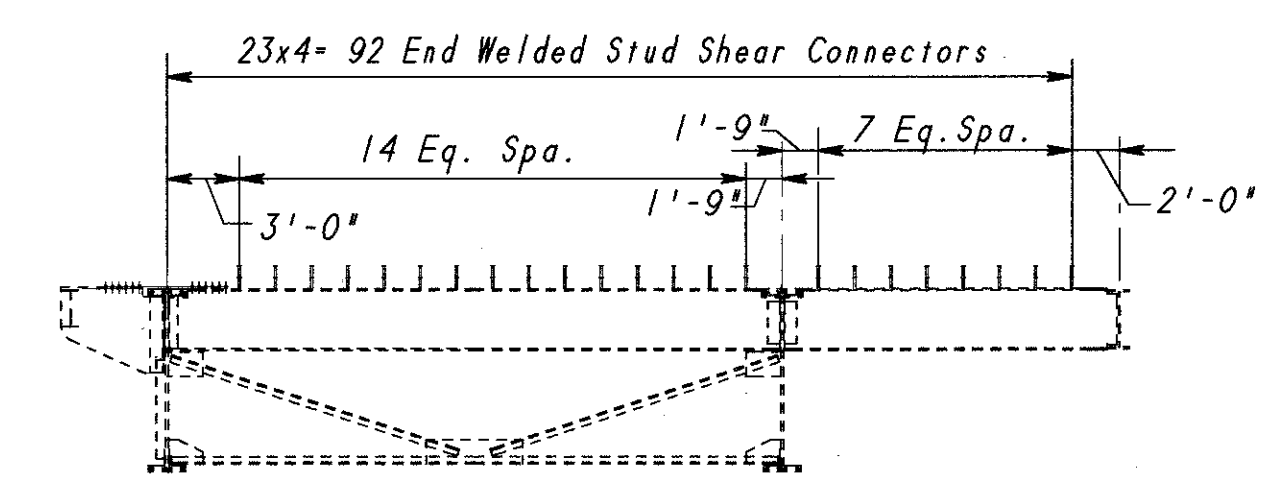


FLOOR BEAM FBH1

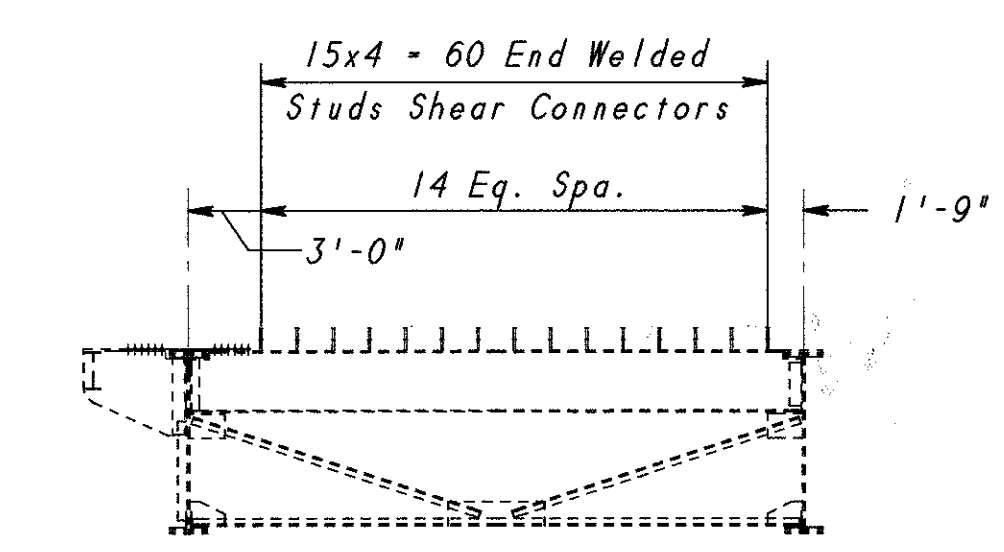
LEGEND

- Existing Structure
- New Work
- FB13 Denotes Floor Beam Unit 1 number 3

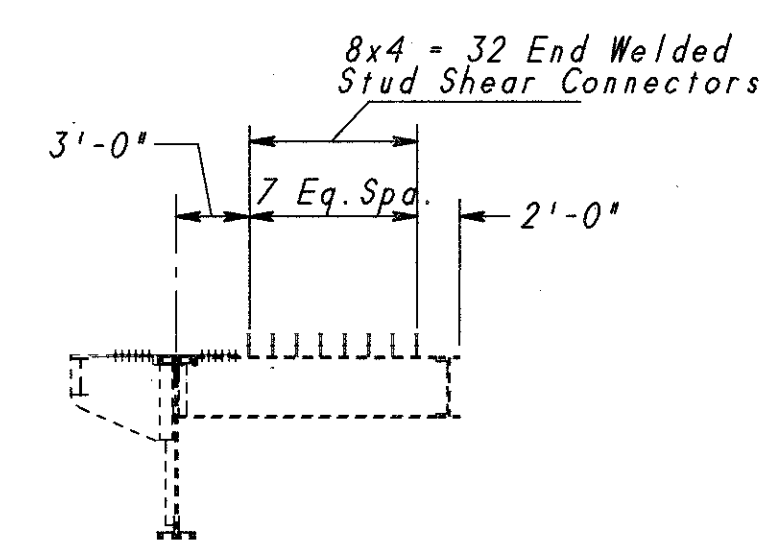
STUDET3 Scale 1/6



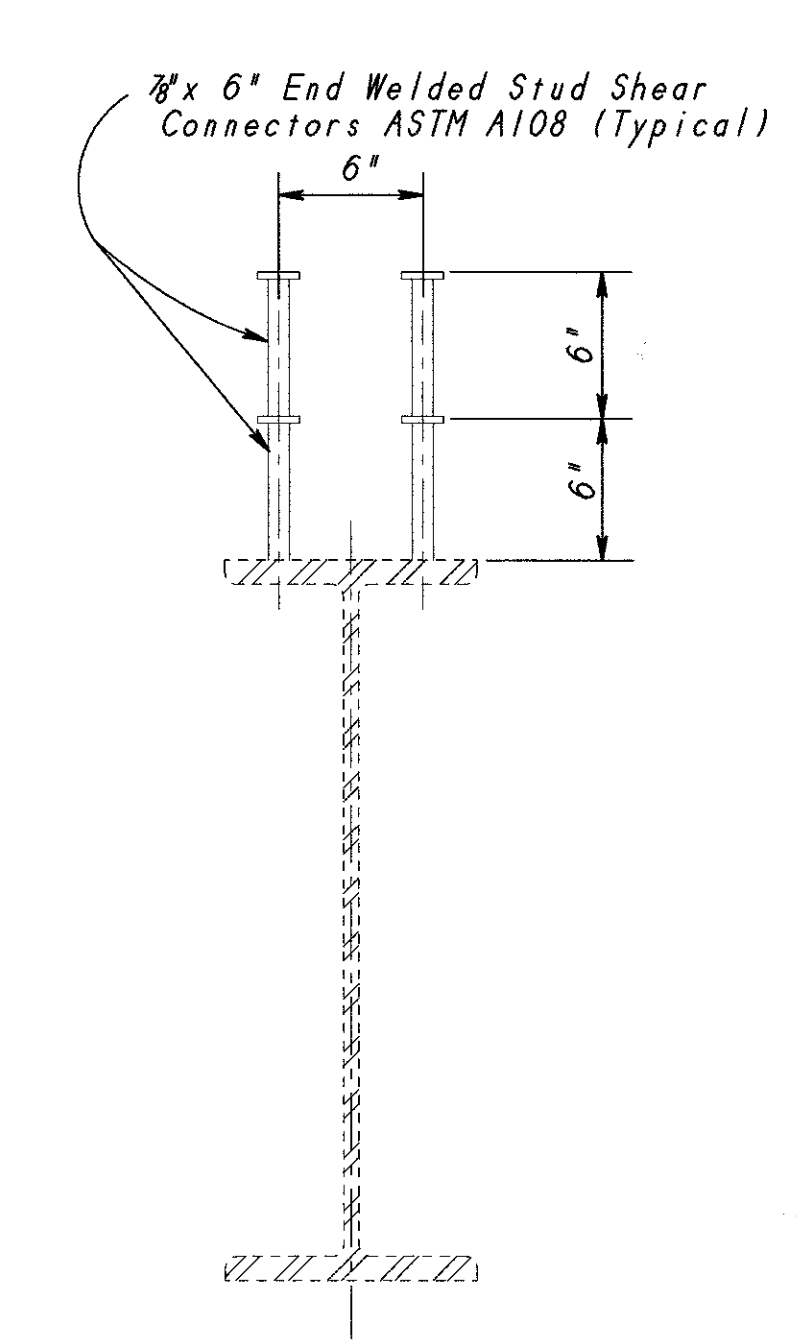
FLOOR BEAM FBH4



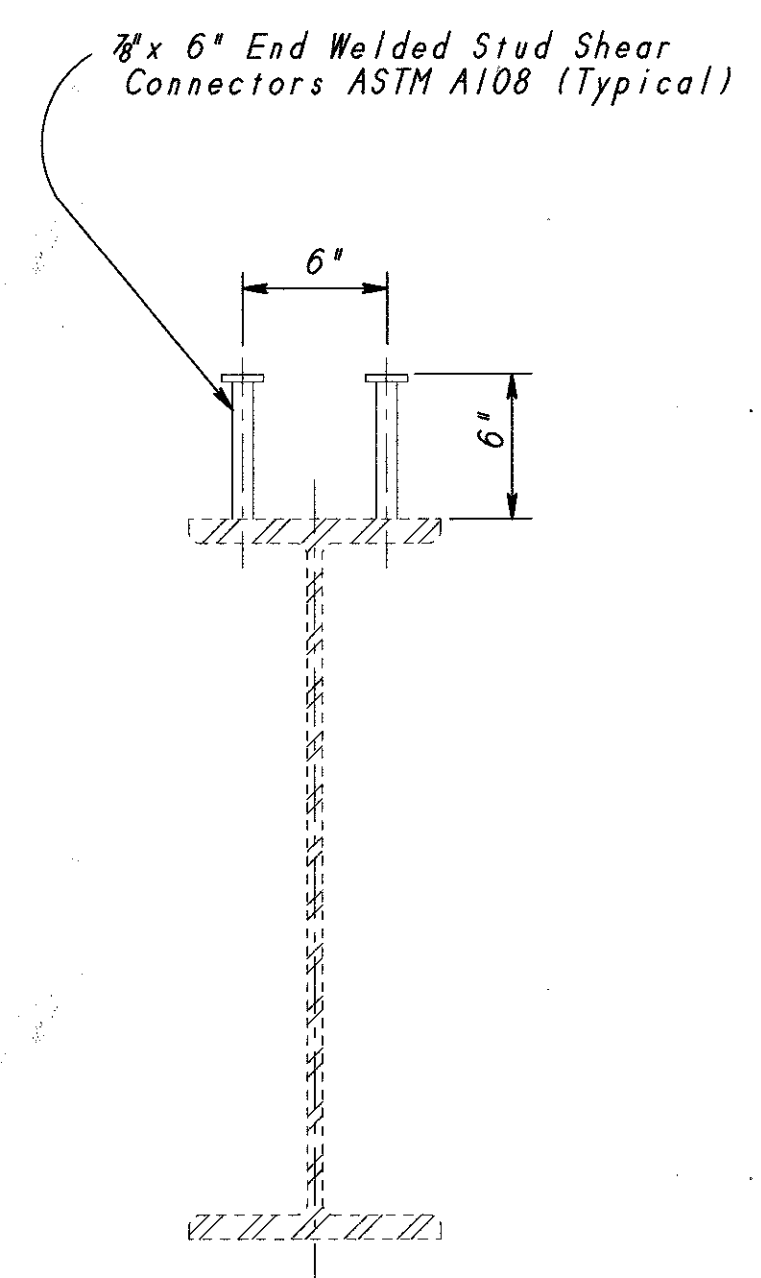
FLOOR BEAM FBH3



FLOOR BEAM FBH2



4 STUDS TYPICAL SECTION



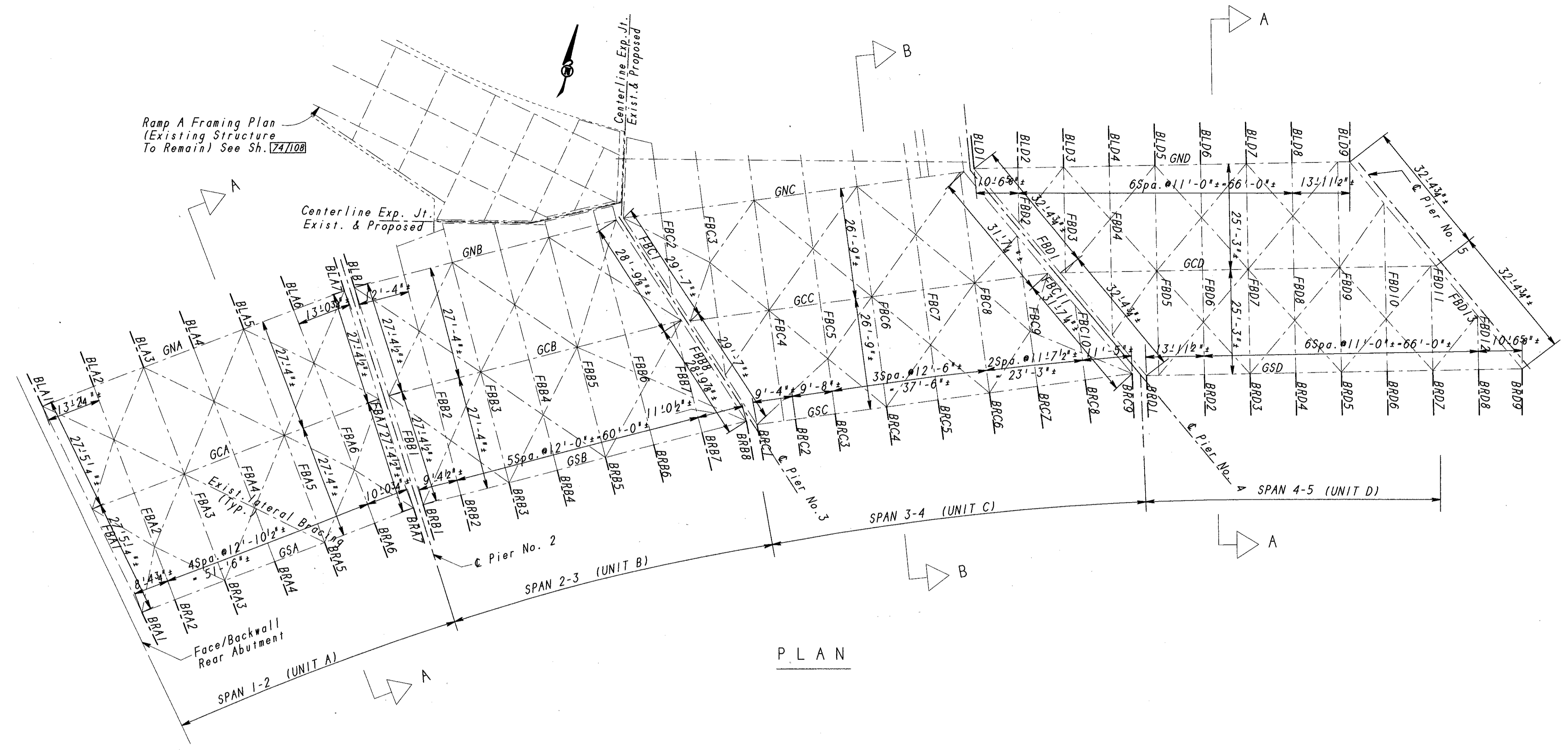
2 STUDS TYPICAL SECTION

STUD SHEAR CONNECTORS CMS 513.171
TOTAL THIS SHEET - 5446

NOTES:

1. - Work this sheet with Sh. 43/108 & 45/108.
2. - All views shown are looking Forward.
3. - See General Notes Sh. 7/108 Thru 11/108.

CINCINNATI		Pfum, Klauemeier & Gehrum Consultants OHIO		40/108
STRUCTURAL STEEL DETAILS STUD SHEAR CONNECTORS SPANS 12-13 THRU SPANS 17-18 (UNITS H THRU M) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY Sra. 30-55.40 Sra. 47-16.19				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
ED	ED/JDR		WDD	IEH April 96



SPAN 1-2 (UNIT A)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLA1	7'-6 3/4"	BRA1	8'-7 3/4"
BLA2	7'-10 1/2"	BRA2	8'-3 1/4"
BLA3	8'-0"	BRA3	7'-10 3/4"
BLA4	7'-11 1/2"	BRA4	7'-9"
BLA5	7'-8 3/4"	BRA5	7'-10 1/2"
BLA6	7'-3 3/4"	BRA6	8'-3"
BLA7	6'-8 1/4"	BRA7	8'-8 3/4"

SPAN 2-3 (UNIT B)

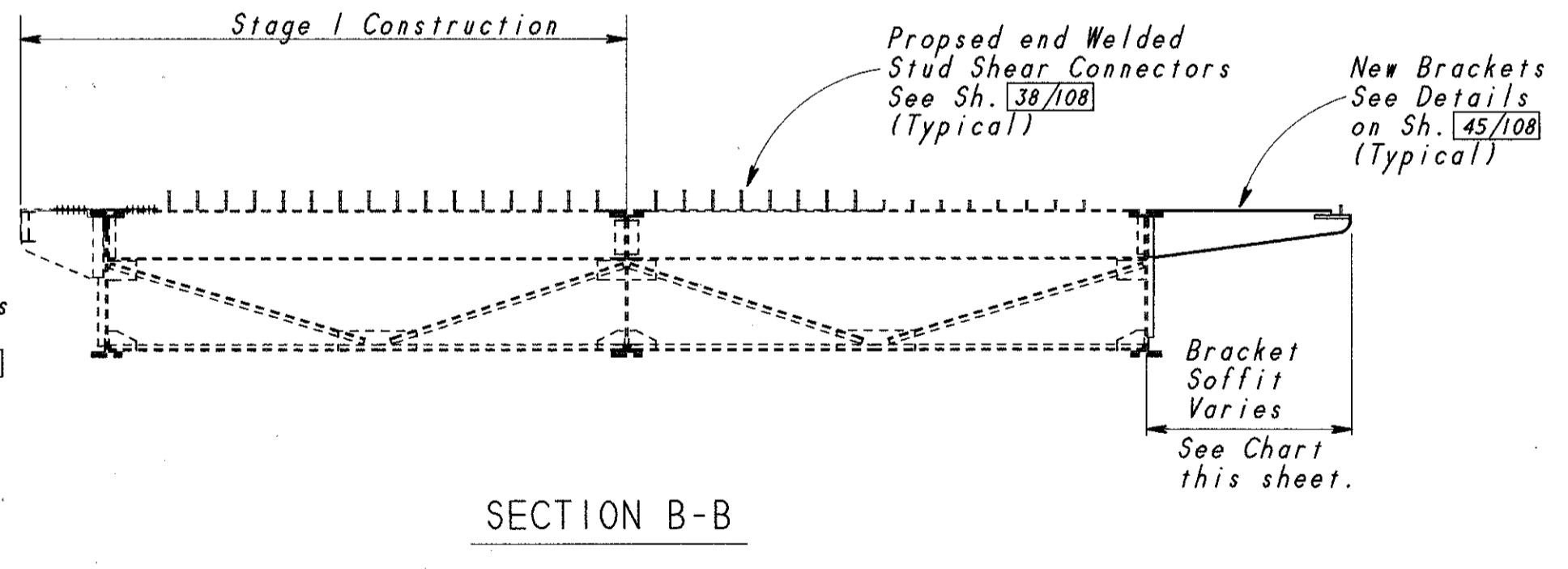
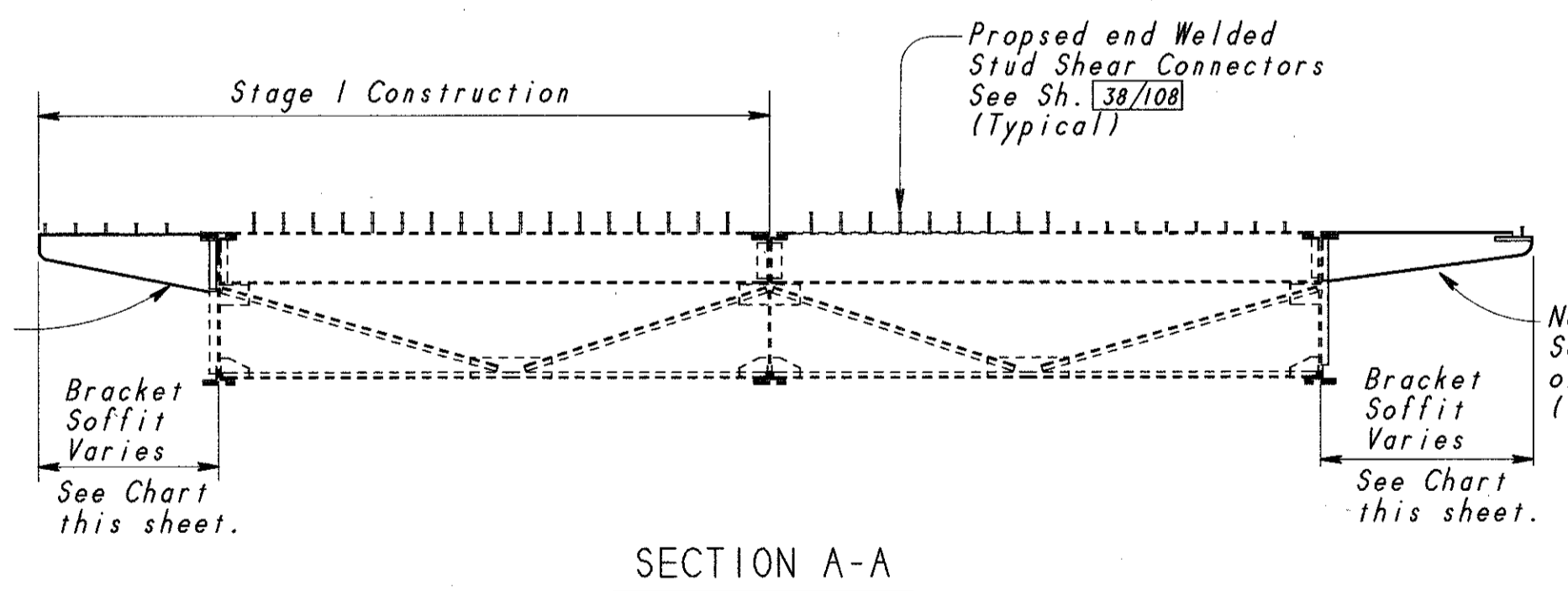
BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLB1	6'-8"	BRB1	8'-8 1/2"
		BRB2	8'-3 1/2"
		BRB3	7'-11 1/4"
		BRB4	7'-9 1/2"
		BRB5	7'-10 3/8"
		BRB6	8'-1 1/8"
		BRB7	8'-8"
		BRB8	9'-3 3/8"

SPAN 3-4 (UNIT C)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
		BRA1	8'-7 3/4"
		BRA2	8'-3 1/2"
		BRA3	8'-0 1/2"
		BRA4	7'-11"
		BRA5	8'-0 5/8"
		BRA6	8'-5"
		BRA7	8'-11 1/2"
		BRA8	9'-8 1/2"
		BRA9	10'-7 1/4"

SPAN 4-5 (UNIT D)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLD1	7'-9 3/8"	BRD1	9'-11 1/2"
BLD2	8'-5 3/8"	BRD2	9'-9 3/4"
BLD3	9'-0"	BRD3	9'-10 3/4"
BLD4	9'-4 3/8"	BRD4	10'-1"
BLD5	9'-7"	BRD5	10'-3 3/8"
BLD6	9'-7 3/8"	BRD6	10'-5 3/8"
BLD7	9'-6 1/4"	BRD7	10'-8"
BLD8	9'-3 3/8"	BRD8	10'-10 3/8"
BLD9	8'-11 3/8"	BRD9	11'-0 3/8"



LEGEND

- BLA1 - Bracket Left Unit A Number 1
 - FBA1 - Floor Beam Unit A Number 1
 - GNA - Girder North Unit A
 - GCA - Girder Center Unit A
 - GSA - Girder South Unit A
 - BRA1 - Bracket Right Unit A Number 1
- Existing Structure
 New Work

NOTES:

1. - Work this sheet with Sh. 42/108 & 43/108, and with Sh. 25/108
2. - See General Notes Sh. 7/108 Thru 11/108

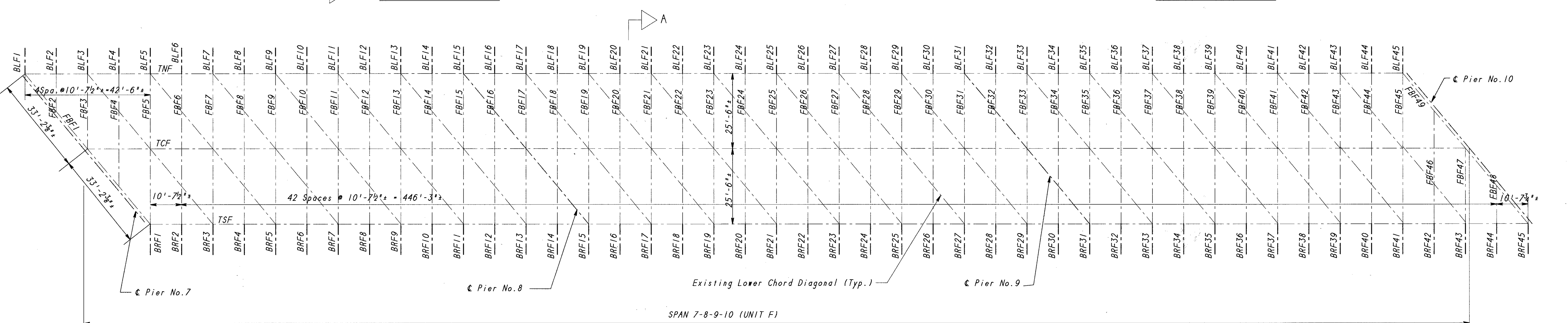
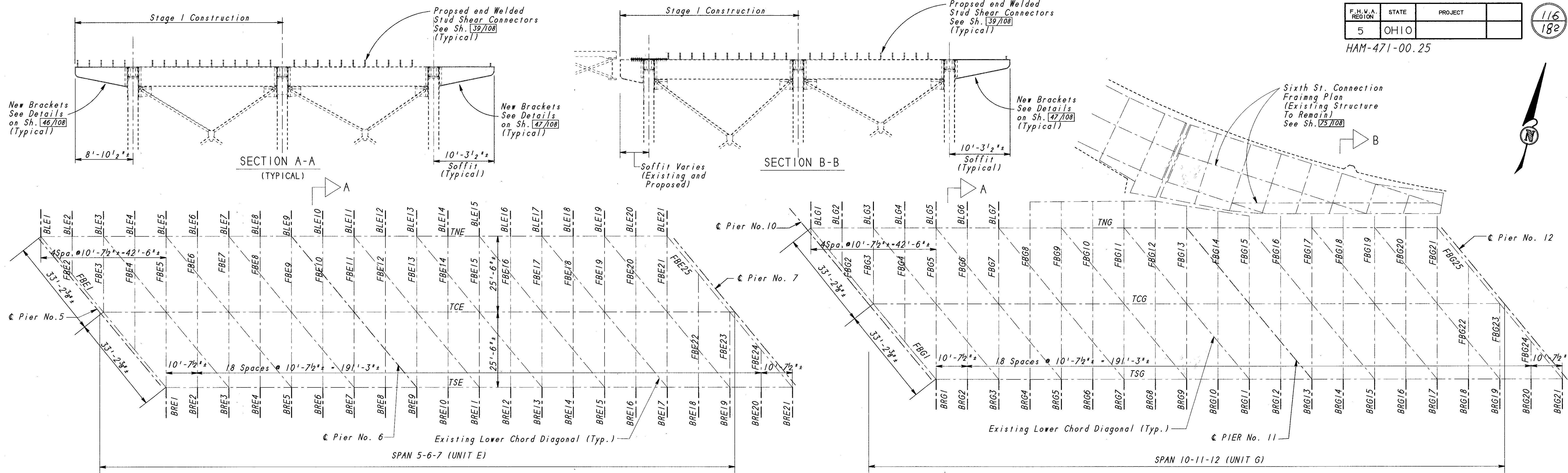
Plum, Klausmeier & Gehrum 41/108
Consultants OHIO

STRUCTURAL STEEL DETAILS
NEW BRACKETS
SPANS 1-2 THRU 4-5
(SLAB UNITS A B C & D)
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)
Sta. 30+55.40
Sta. 47+16.19

HAMILTON COUNTY

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED/JDR		WDD	IEH	April 96	

STLABCD Scale 1/6



LEGEND

- BLF1 - Bracket Left Unit F Number 1
- FBF1 - Floor Beam Unit F Number 1
- TNF - Truss North Unit F
- TCF - Truss Center Unit F
- TSF - Truss South Unit F
- BRF1 - Bracket Right Unit F Number 1

Existing Structure (dashed line)
 New Work (solid line)

- NOTES:**
- Work this sheet with Sh. 41/108 & Sh. 43/108 and with Sh. 25/108 and Sh. 26/108.
 - See General Notes Sh. 7/108 Thru 11/108.

Plum, Klausmeyer & Gehrmann
 CONSULTANTS
 42/108
 OHIO

STRUCTURAL STEEL DETAILS
 NEW BRACKETS
 TRUSS SPANS 5-6-7, 7-8-9-10, & 10-11-12
 (SLAB UNITS E & G)
 BRIDGE No. HAM-471-0025
 (COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
 Sta. 30+55.40
 Sta. 47+16.19

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISION
ED	ED/JDR		WDD	IEH	April 96	

STL/EG Scale 1/8"

SPAN 12-13 (UNIT H)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
		BRH1	9'-10 1/4"
		BRH2	9'-10 1/2"
		BRH3	9'-9 3/4"
		BRH4	9'-7 3/4"
		BRH5	9'-4 5/8"

SPAN 14-15 (UNIT J)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLJ1	8'-7 1/2"	BRJ1	9'-1 3/4"
BLJ2	8'-3 1/2"	BRJ2	9'-7"
BLJ3	7'-11 1/2"	BRJ3	9'-10 5/8"
BLJ4	7'-9 1/4"	BRJ4	10'-0 3/4"
BLJ5	7'-8 1/2"	BRJ5	10'-1 1/2"
BLJ6	7'-9 1/4"	BRJ6	10'-0 3/4"
BLJ7	7'-11 1/2"	BRJ7	9'-10 3/4"
BLJ8	8'-3 3/8"	BRJ8	9'-7 1/4"
BLJ9	8'-7 1/4"	BRJ9	9'-2"

SPAN 16-17 (UNIT L)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLL1	8'-5 3/8"	BRL1	9'-4 1/8"
BLL2	8'-2"	BRL2	9'-8 1/2"
BLL3	7'-11 1/2"	BRL3	9'-10 1/2"
BLL4	7'-10 3/4"	BRL4	9'-11 1/4"
BLL5	7'-11 3/8"	BRL5	9'-10 3/8"
BLL6	8'-2 1/8"	BRL6	9'-8"
BLL7	8'-5 1/2"	BRL7	9'-4"

SPAN 17-18 (UNIT M)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLM1	8'-5"	BRM1	9'-4 1/2"
BLM2	8'-2"	BRM2	9'-8 1/4"
BLM3	7'-11 3/4"	BRM3	9'-10 1/4"
BLM4	7'-11 1/8"	BRM4	9'-10 3/4"
BLM5	8'-0 1/4"	BRM5	9'-9 3/4"
BLM6	8'-2 3/4"	BRM6	9'-7 1/4"
BLM7	8'-5 1/8"	BRM7	9'-3"

SPAN 13-14 (UNIT I)

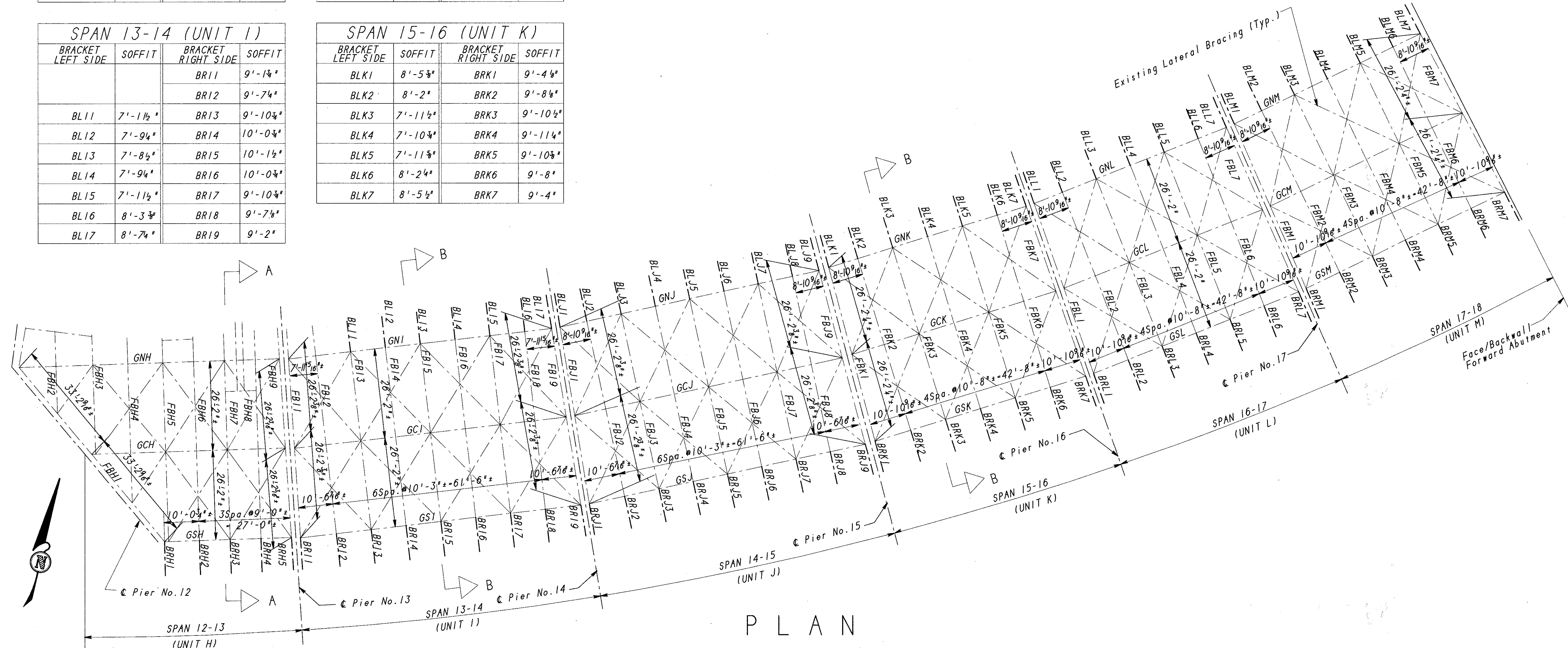
BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
		BR11	9'-1 1/4"
		BR12	9'-7 1/4"
BL11	7'-11 1/2"	BR13	9'-10 3/4"
BL12	7'-9 1/4"	BR14	10'-0 3/4"
BL13	7'-8 1/2"	BR15	10'-1 1/2"
BL14	7'-9 1/4"	BR16	10'-0 3/4"
BL15	7'-11 1/2"	BR17	9'-10 3/4"
BL16	8'-3 3/8"	BR18	9'-7 1/8"
BL17	8'-7 1/4"	BR19	9'-2"

SPAN 15-16 (UNIT K)

BRACKET LEFT SIDE	SOFFIT	BRACKET RIGHT SIDE	SOFFIT
BLK1	8'-5 3/8"	BRK1	9'-4 1/8"
BLK2	8'-2"	BRK2	9'-8 1/2"
BLK3	7'-11 1/2"	BRK3	9'-10 1/2"
BLK4	7'-10 3/4"	BRK4	9'-11 1/4"
BLK5	7'-11 3/8"	BRK5	9'-10 3/8"
BLK6	8'-2 1/8"	BRK6	9'-8"
BLK7	8'-5 1/2"	BRK7	9'-4"

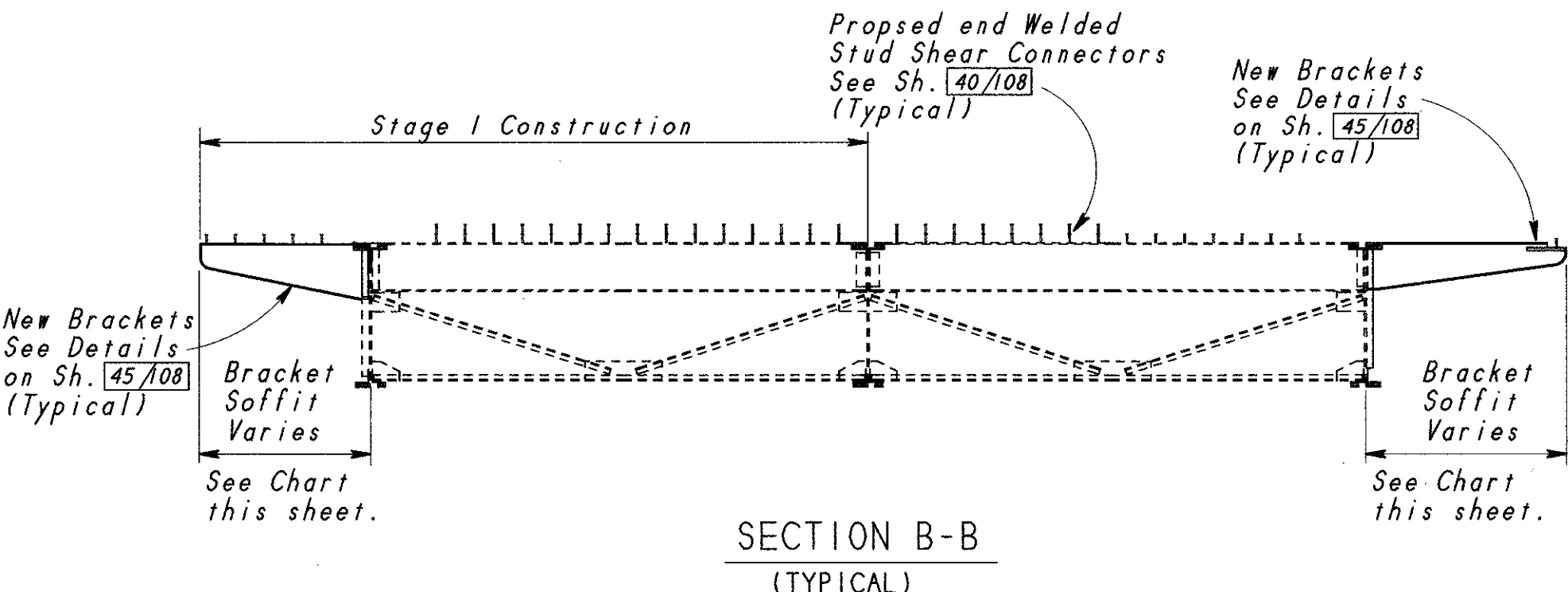
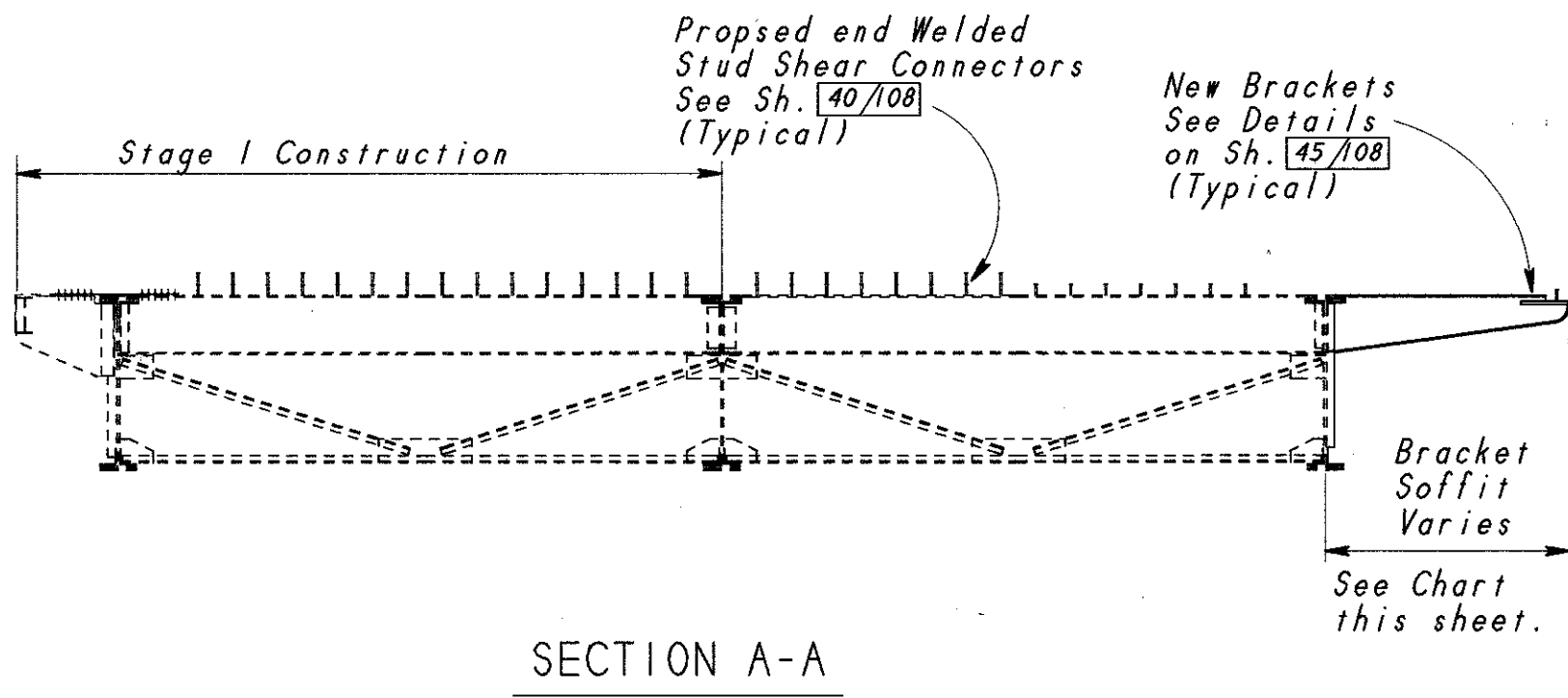
NOTES:

- Work this sheet with Sh. 41/108 & Sh. 42/108. With Sh. 26/108 & 27/108.
- See General Notes Sh. 7/108 Thru 11/108.



PLAN

- LEGEND**
- BLJ1 - Bracket Left Unit J Number 1
 - FBH1 - Floor Beam Unit H Number 1
 - GNH - Girder North Unit H
 - GCH - Girder Center Unit H
 - GSH - Girder South Unit H
 - BRH1 - Bracket Right Unit H Number 1
 - Existing Structure (dashed line)
 - New Work (solid line)



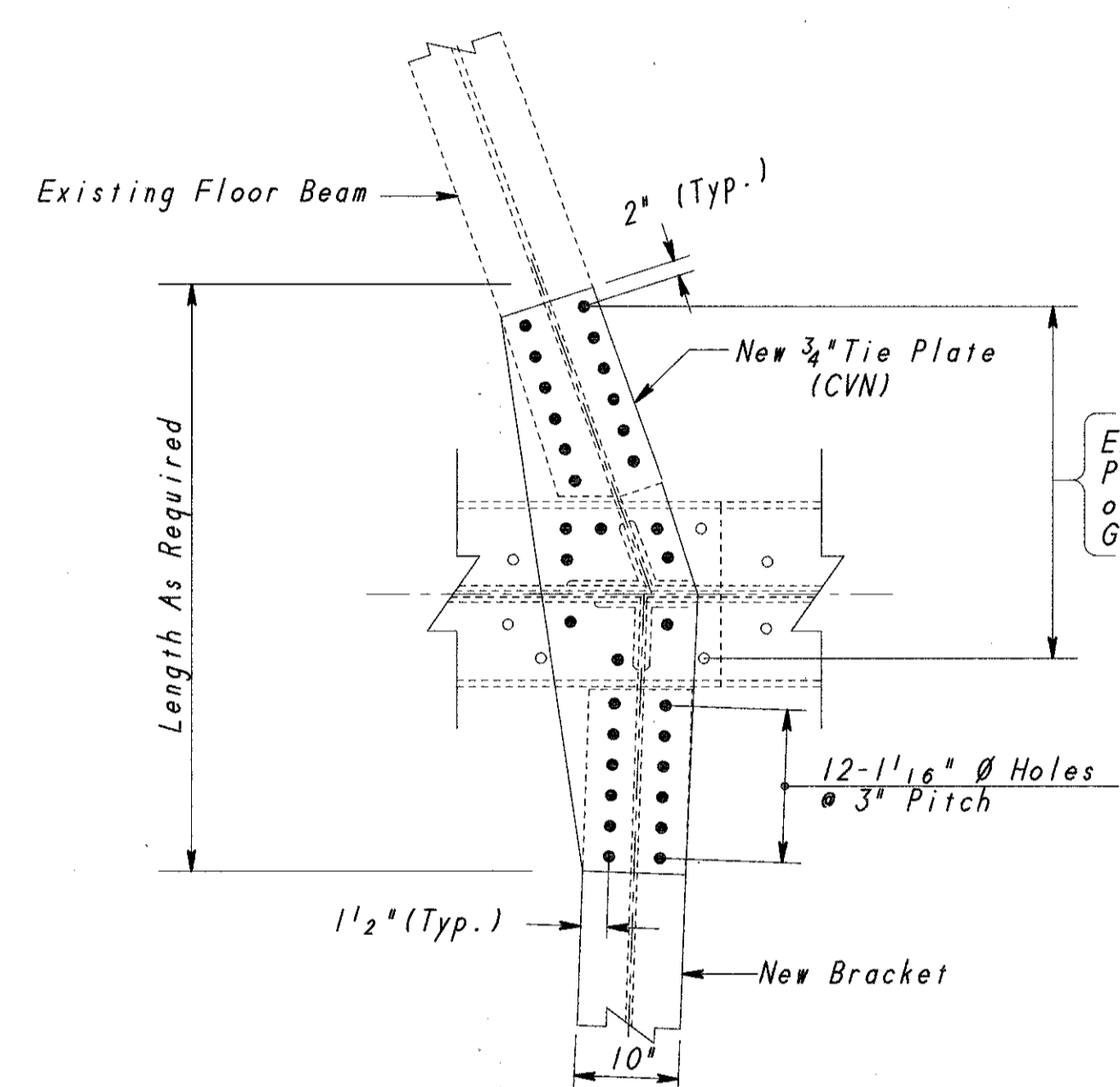
STRUCTURAL STEEL DETAILS
NEW BRACKETS
SPANS 12-13 THRU 17-18
(SLAB UNITS H, I, J, K, L, & M)
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

Hamilton County
DESIGNED: ED / JDR
DRAWN: ED / JDR
TRACED: WDD
CHECKED: WDD
REVIEWED: IEH April 96
DATE: 4/96

Plum, Klausmeier & Gehrum
CONSULTANTS
43/108
OHIO

Sta. 30+55.40
Sta. 47+16.19

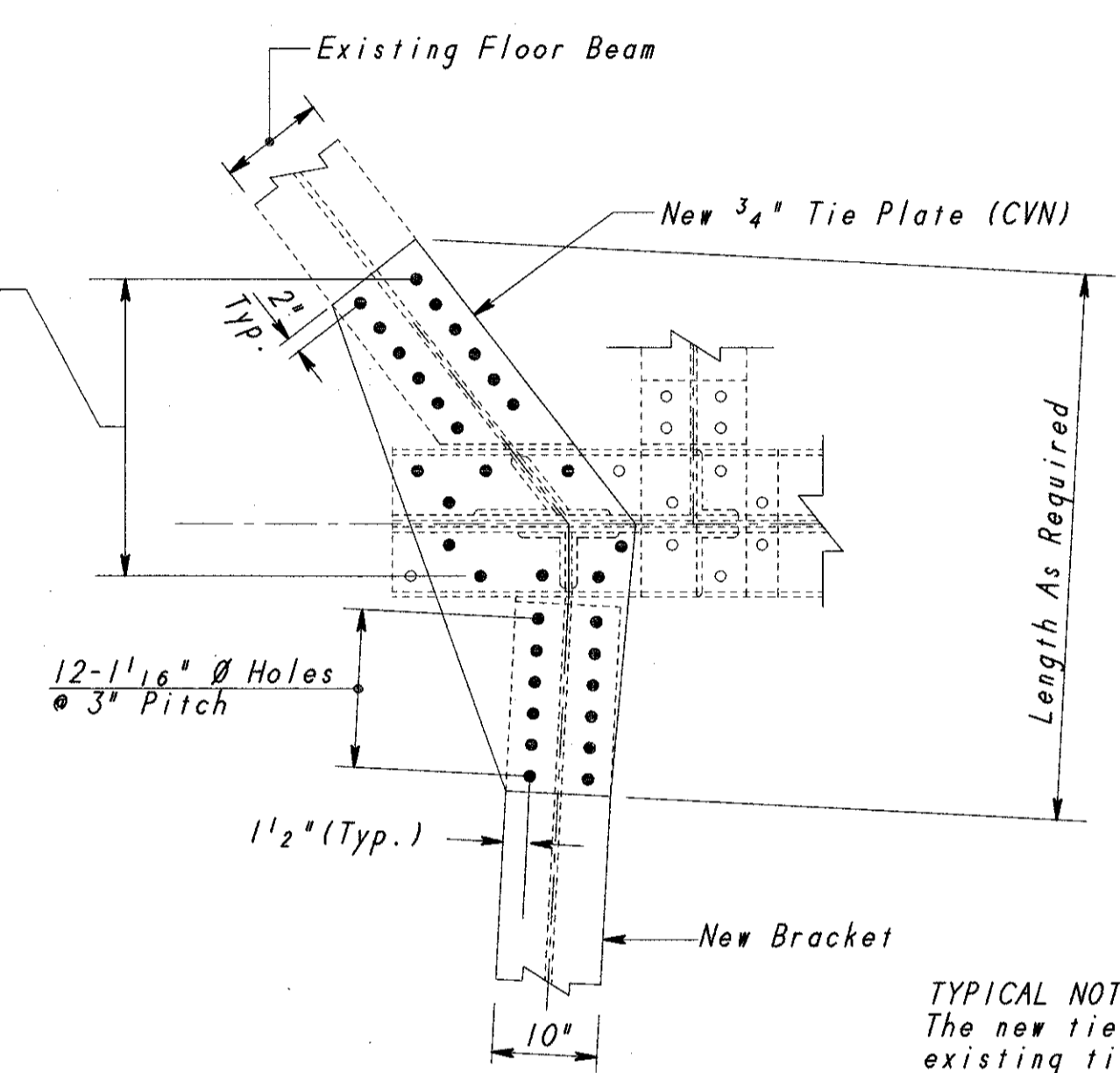
STLH-M Scale 1:6



NEW BRACKET BRB8, BRC1, & BRC9
NEW BRACKET BLD1, BRD1, BRD9, & BLD9

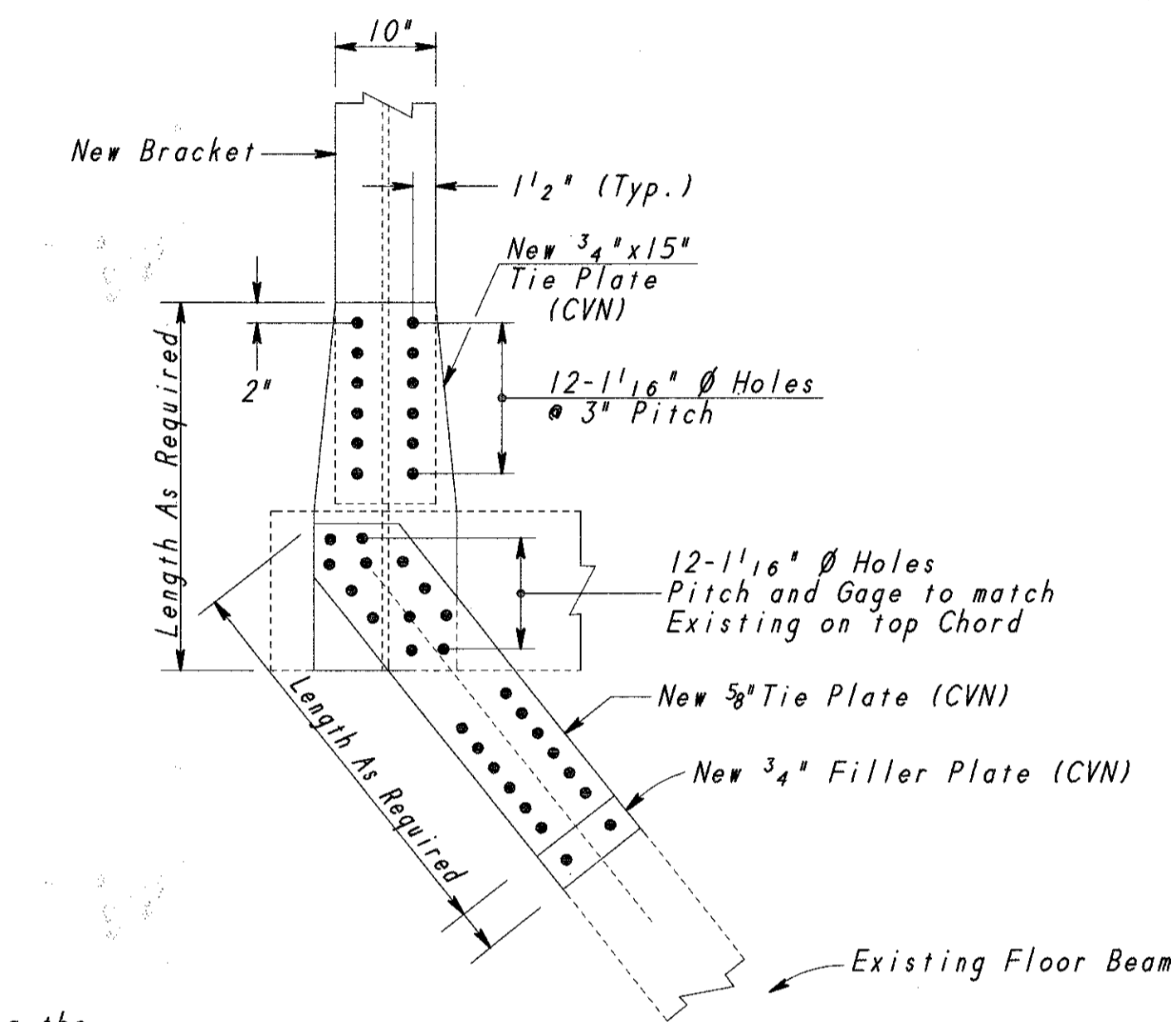
21-1 1/16" Ø Holes (Match Existing Pitch and Gage On Floor Beam Top Flange And Girder Top Flange). Typical.

Existing holes. Ream to 1 1/16" Ø. Pitch and Gage same as existing on Floor Beam Top Flange, and Girder Top Flange (Typical).



NEW BRACKET BRH1

TYPICAL NOTE.-
The new tie plates shown shall be cut using the existing tie plates to be removed and replaced, as template.



NEW BRACKETS BRE1, BRE21, BRF1, BRF45, BRG1, BRG21, BLE1, BLE21, BLF1, BLF45, & BLG1.

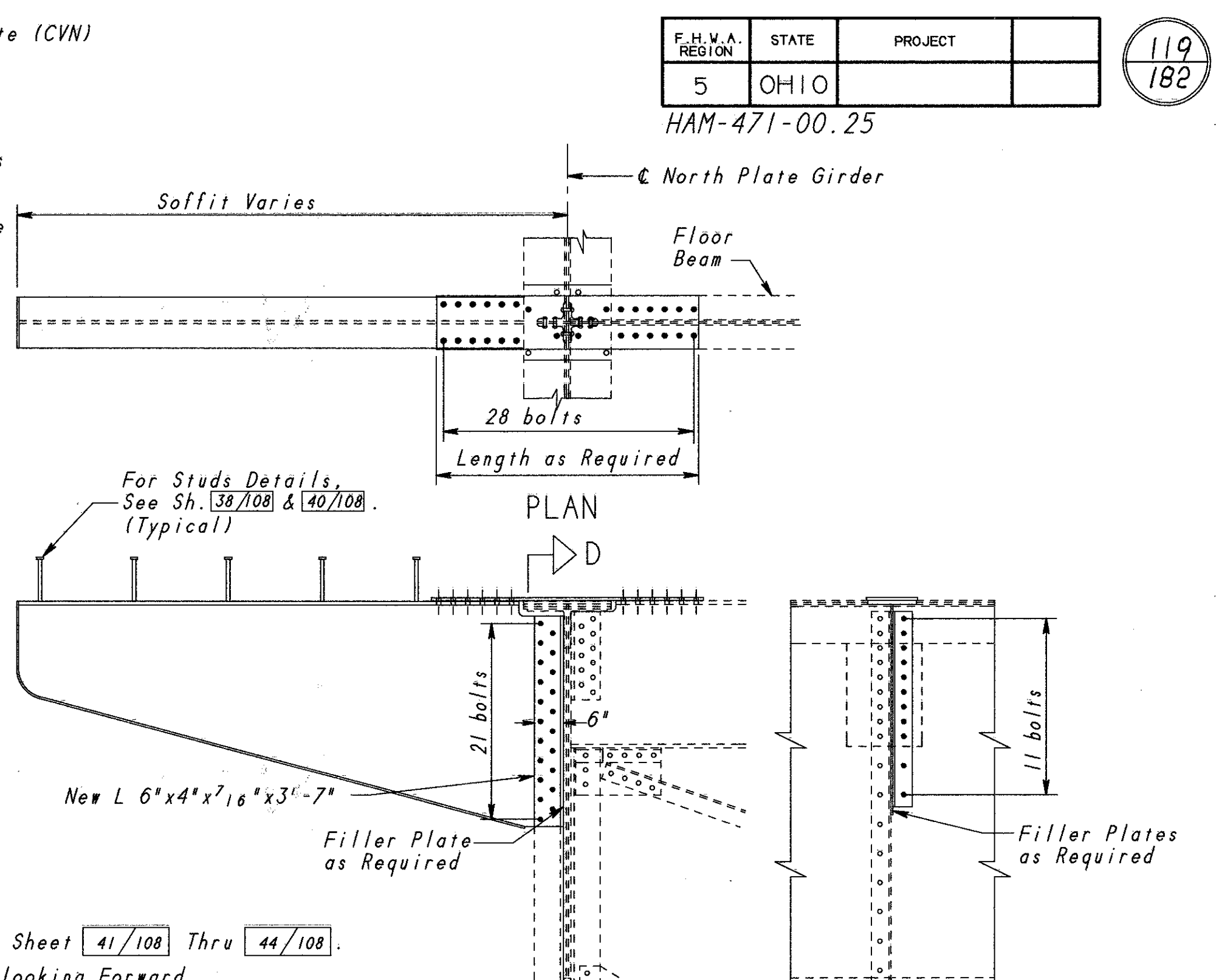
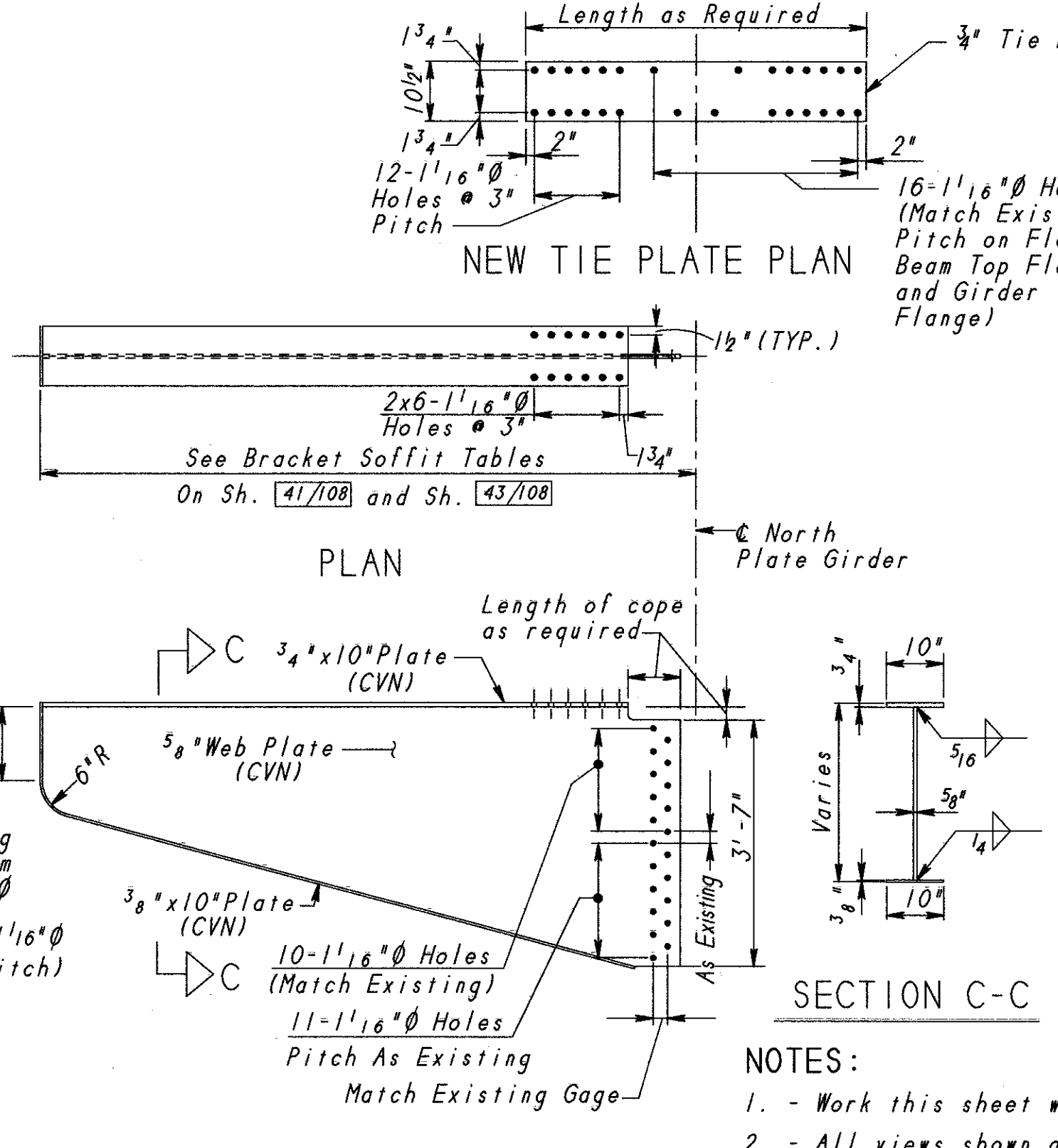
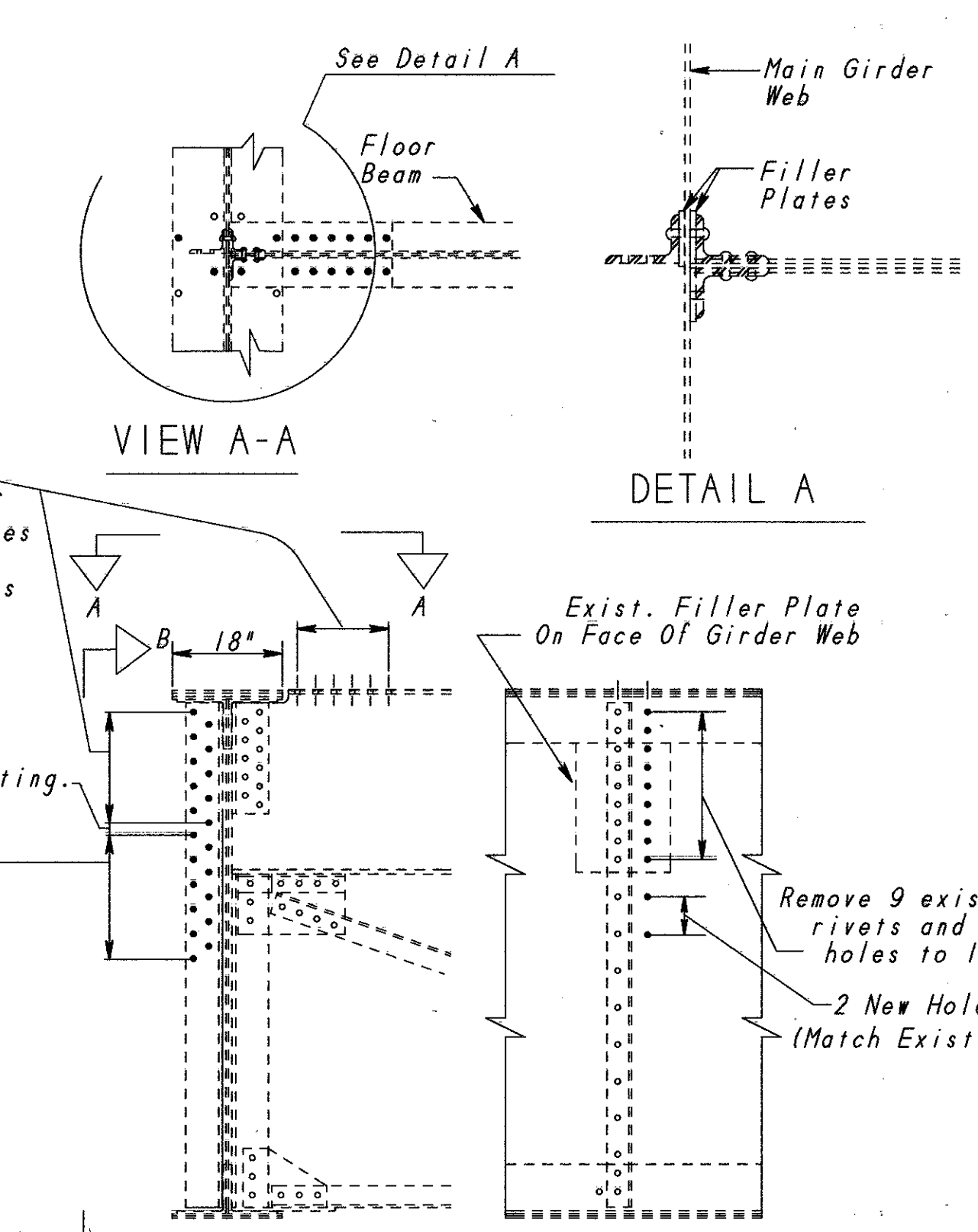
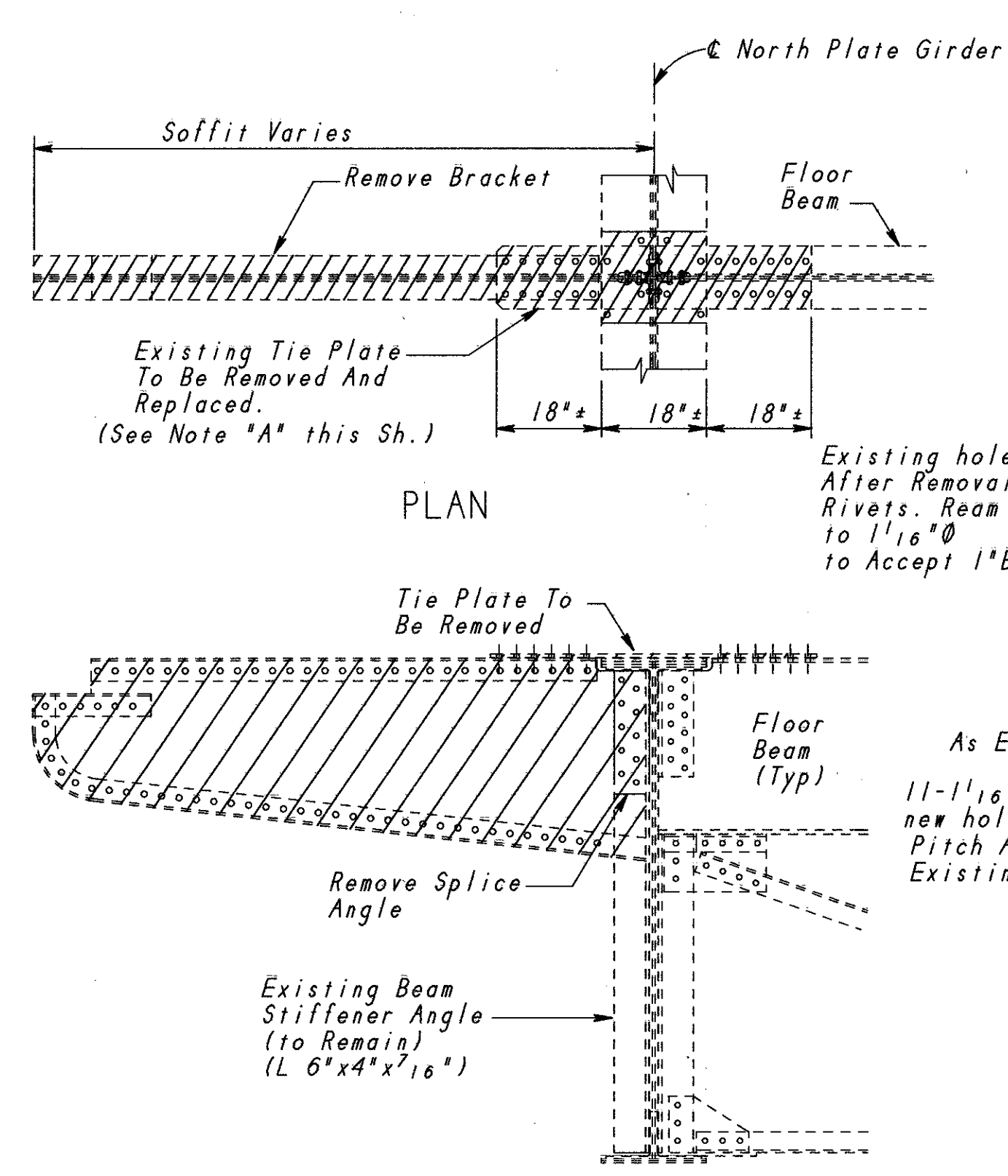
LEGEND

- BLA1 - Bracket Left Unit A Number 1
- BRA1 - Bracket Right Unit A Number 1
- Existing Structure
- New Work
- Existing Hole
- New Hole Or Reamed Hole

NOTES:

1. Work this sheet with sheets 41/108, 42/108, 43/108, 45/108 and 46/108
2. Representation Of Existing Structure Is Approximate.
3. See Note "A" on Sh. 45/108.

Pflum, Klausmeier & Gohrum Consultants		44 / 108
STRUCTURAL STEEL DETAILS SPECIAL TIE PLATES ON NEW BRACKETS SPANS 1 - 2 THRU 17 - 18 (UNITS A THRU M) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
	IEH	April 96



LEGEND

Existing Structure

New Work

New Hole Or Reamed Hole

Existing Hole

VIEW A-A
VIEW B-B

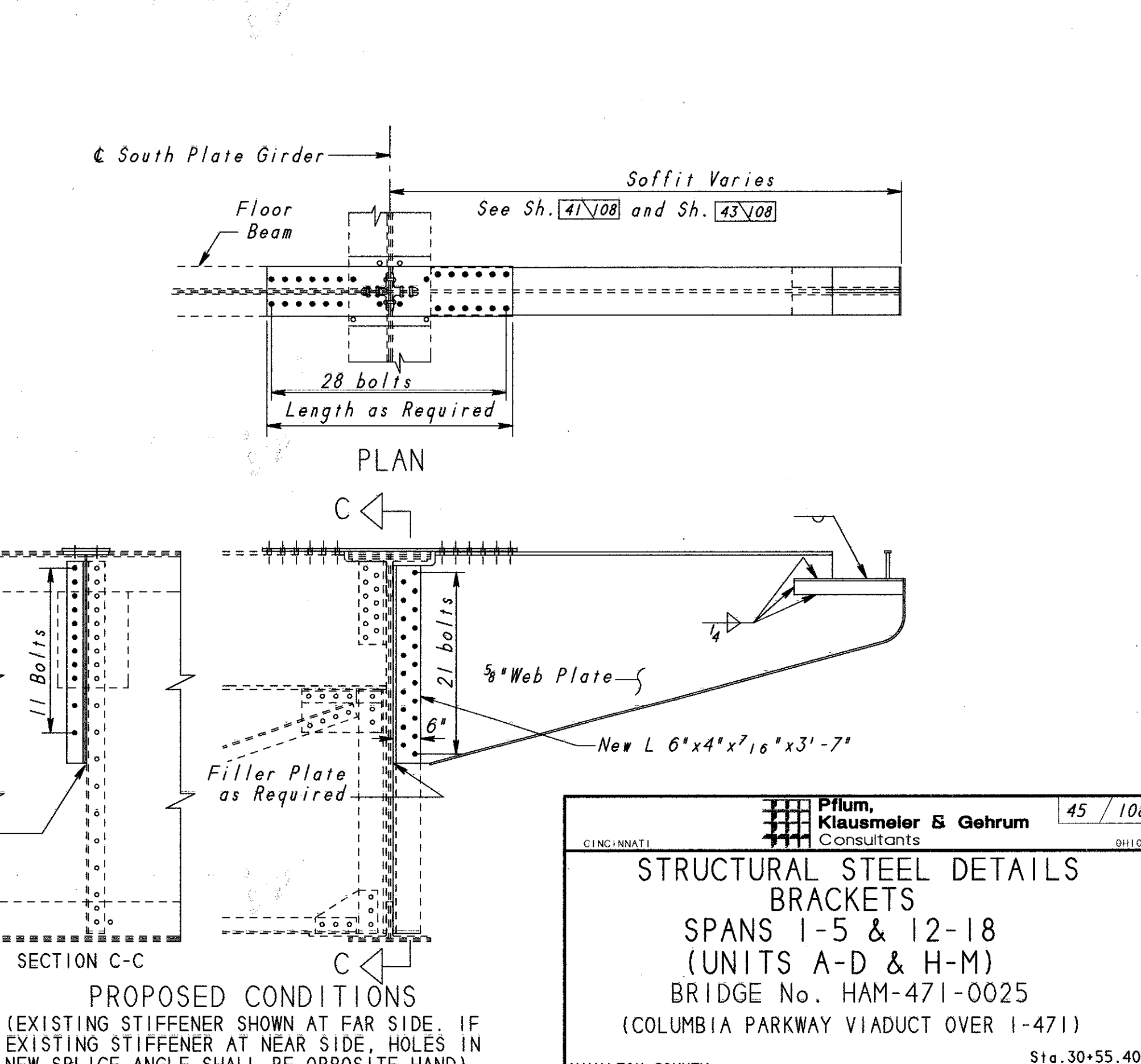
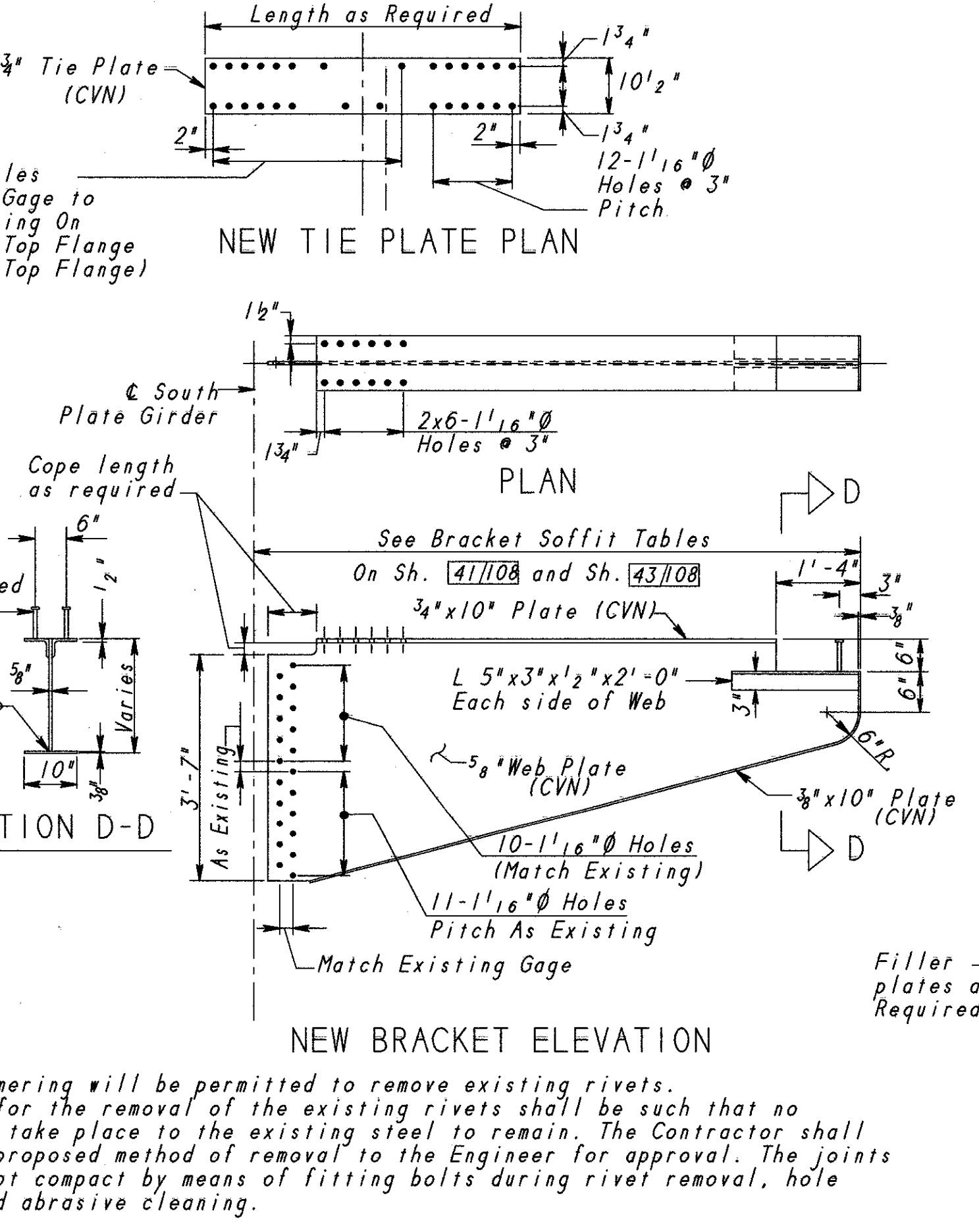
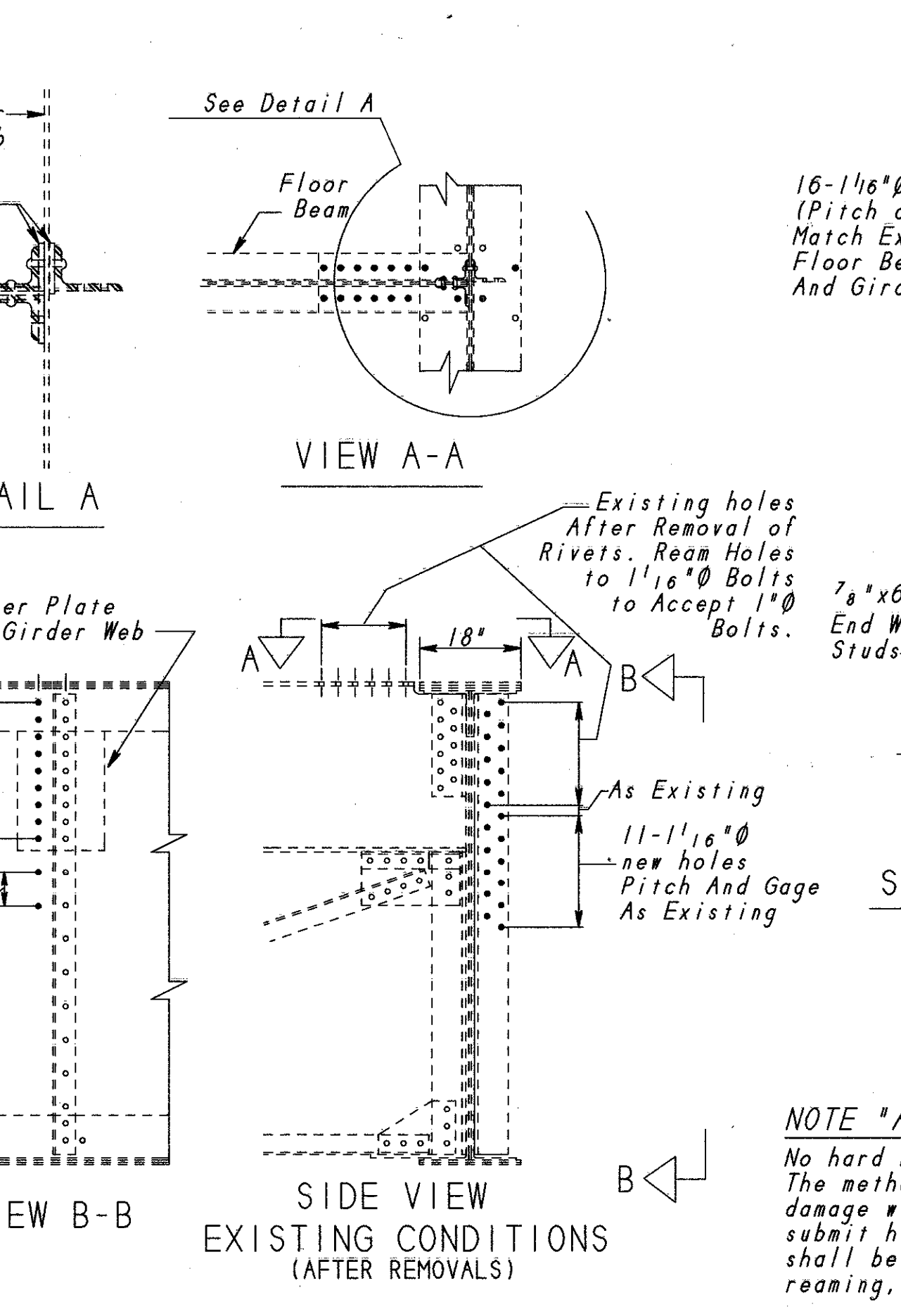
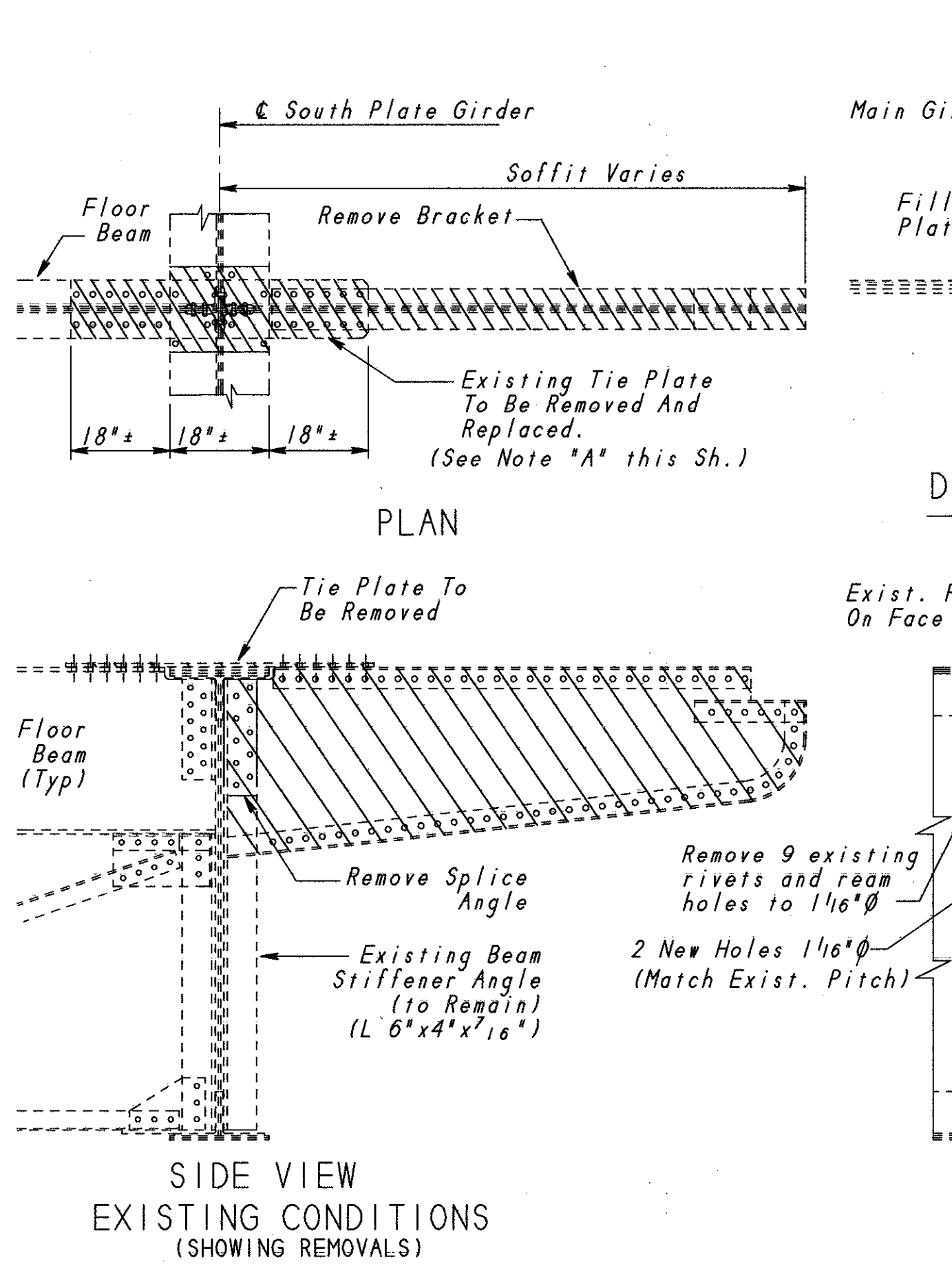
NEW TIE PLATE PLAN
NEW BRACKET DETAIL

PLAN
ELEVATION
SECTION D-D

- NOTES:**
1. - Work this sheet with Sheet 41/108 Thru 44/108.
 2. - All views shown are looking Forward.
 3. - See General Notes Sh. 7/108 Thru 11/108.
 4. - All bolts 1" ASTM A325
 5. - Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in CMS 711.01

TYPICAL LEFT SIDE BRACKET REPLACEMENT SCHEDULE: SPANS 1-2 THRU SPANS 4-5 AND SPANS 12-13 THRU SPANS 17-18
(Representation Of Existing Structure Is Approximate)

PROPOSED CONDITIONS
(EXISTING STIFFENER SHOWN AT FAR SIDE, IF EXISTING STIFFENER AT NEAR SIDE, HOLES IN NEW SPLICE ANGLE SHALL BE OPPOSITE HAND)



NOTE "A"
No hard hammering will be permitted to remove existing rivets. The method for the removal of the existing rivets shall be such that no damage will take place to the existing steel to remain. The Contractor shall submit his proposed method of removal to the Engineer for approval. The joints shall be kept compact by means of fitting bolts during rivet removal, hole reaming, and abrasive cleaning.

TYPICAL RIGHT SIDE BRACKET REPLACEMENT SCHEDULE: SPANS 1-2 THRU SPANS 4-5 AND SPANS 12-13 THRU SPANS 17-18
(Representation Of Existing Structure Is Approximate)

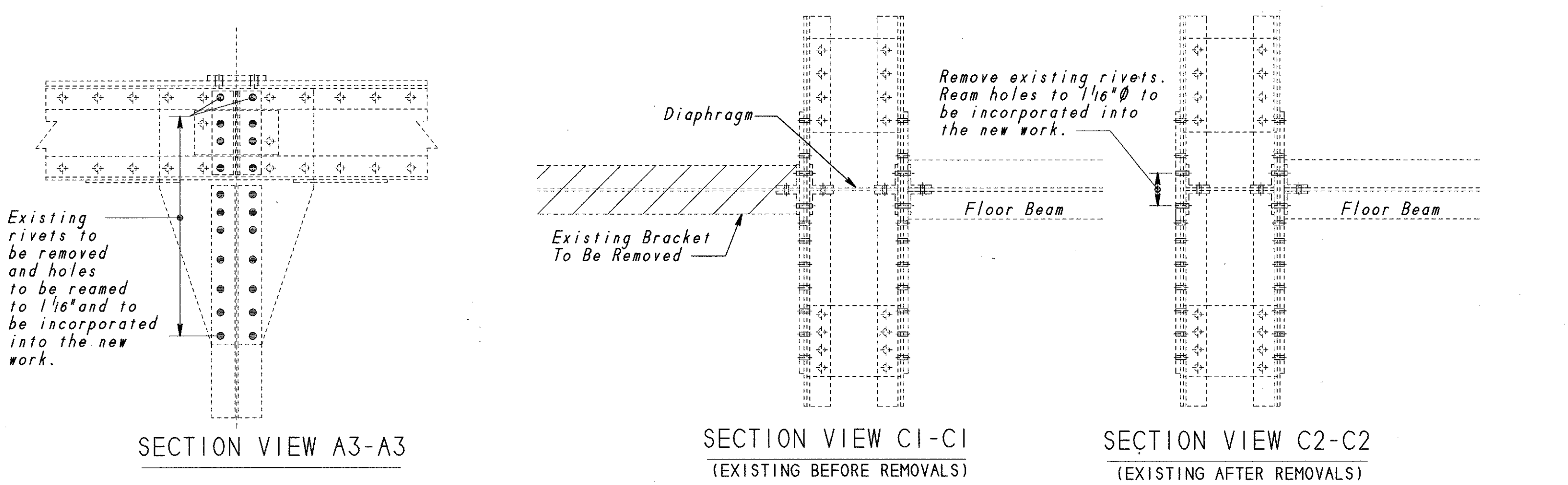
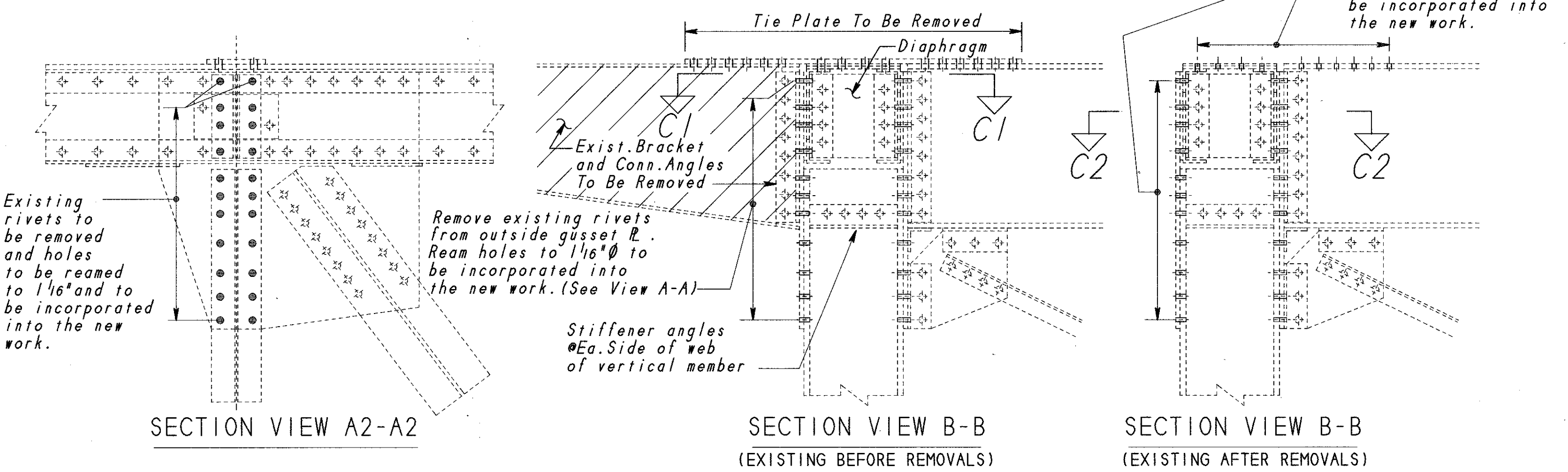
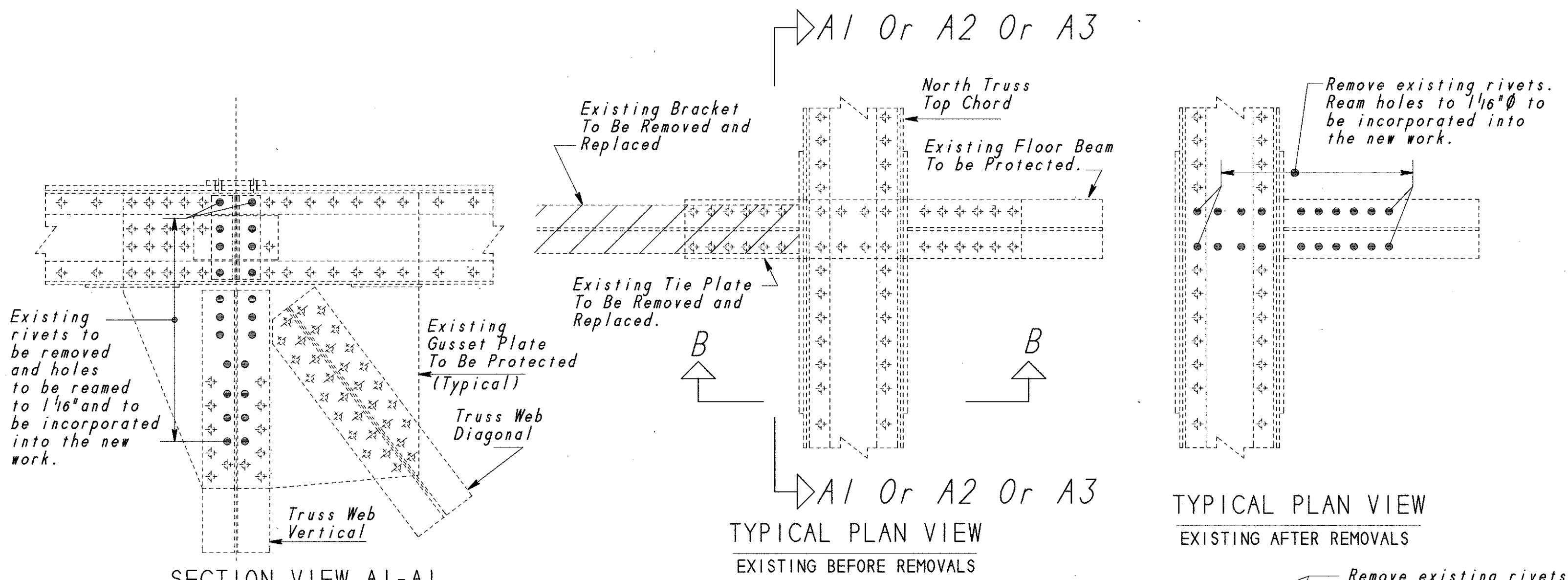
PROPOSED CONDITIONS
(EXISTING STIFFENER SHOWN AT FAR SIDE, IF EXISTING STIFFENER AT NEAR SIDE, HOLES IN NEW SPLICE ANGLE SHALL BE OPPOSITE HAND)

Plum, Klausmeier & Gehrmann Consultants		45 / 108
STRUCTURAL STEEL DETAILS BRACKETS SPANS 1-5 & 12-18 (UNITS A-D & H-M) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

EXBRACK Scale 2

HAM-471-00.25

- NOTES:
1. - Work This Sheet With Sh. 41/108 Thru 44/108.
 2. - All Views Shown Are Looking Forward.
 3. - See General Notes Sh. 7/108 Thru 11/108.
 4. - All bolts 1" H.S. ASTM A325
 5. - Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in CMS 711.01



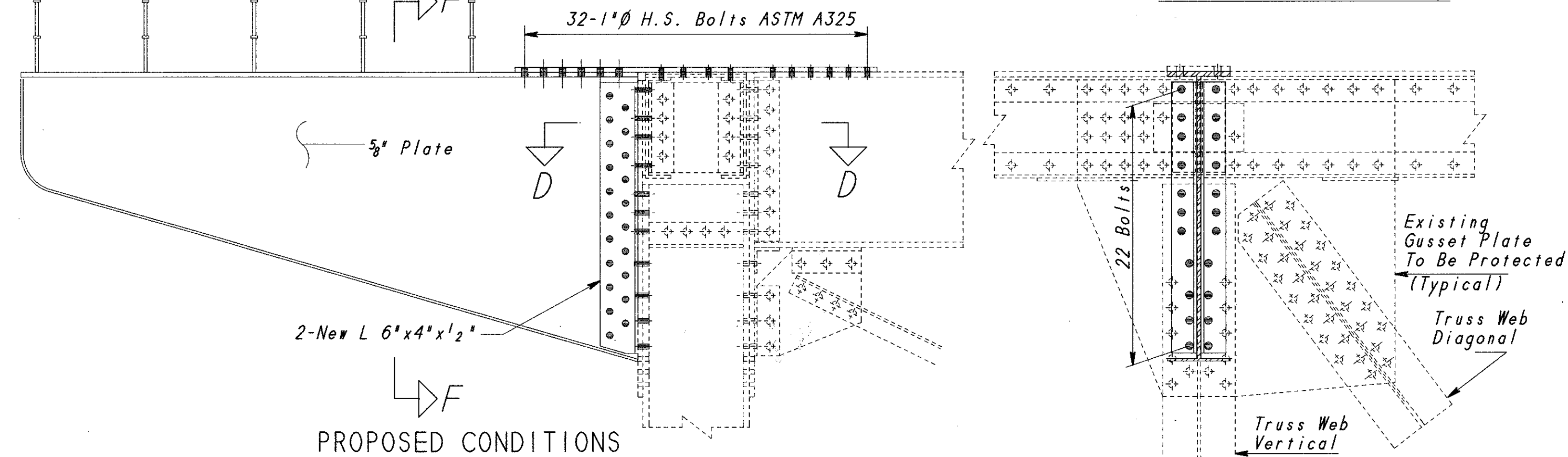
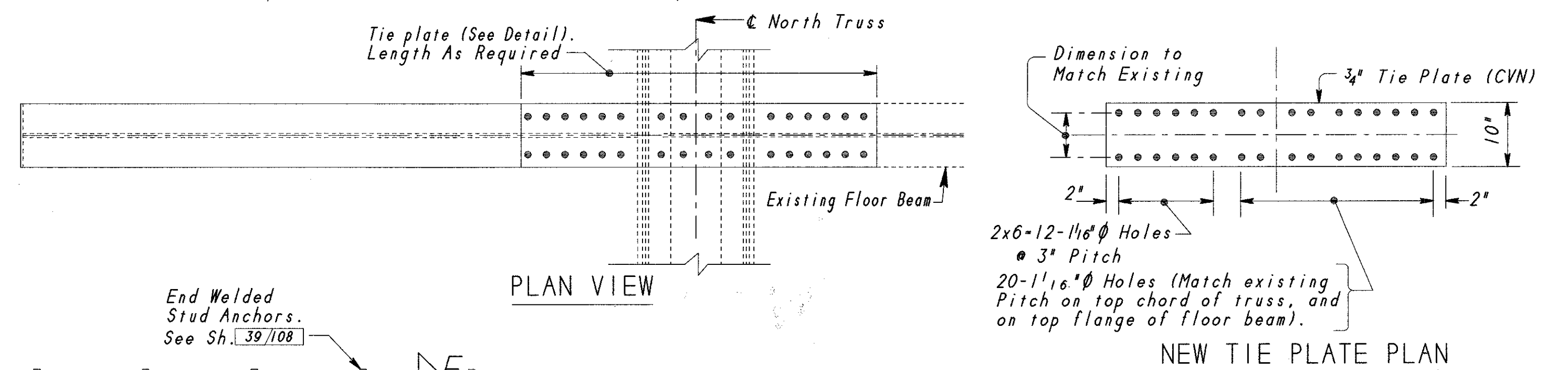
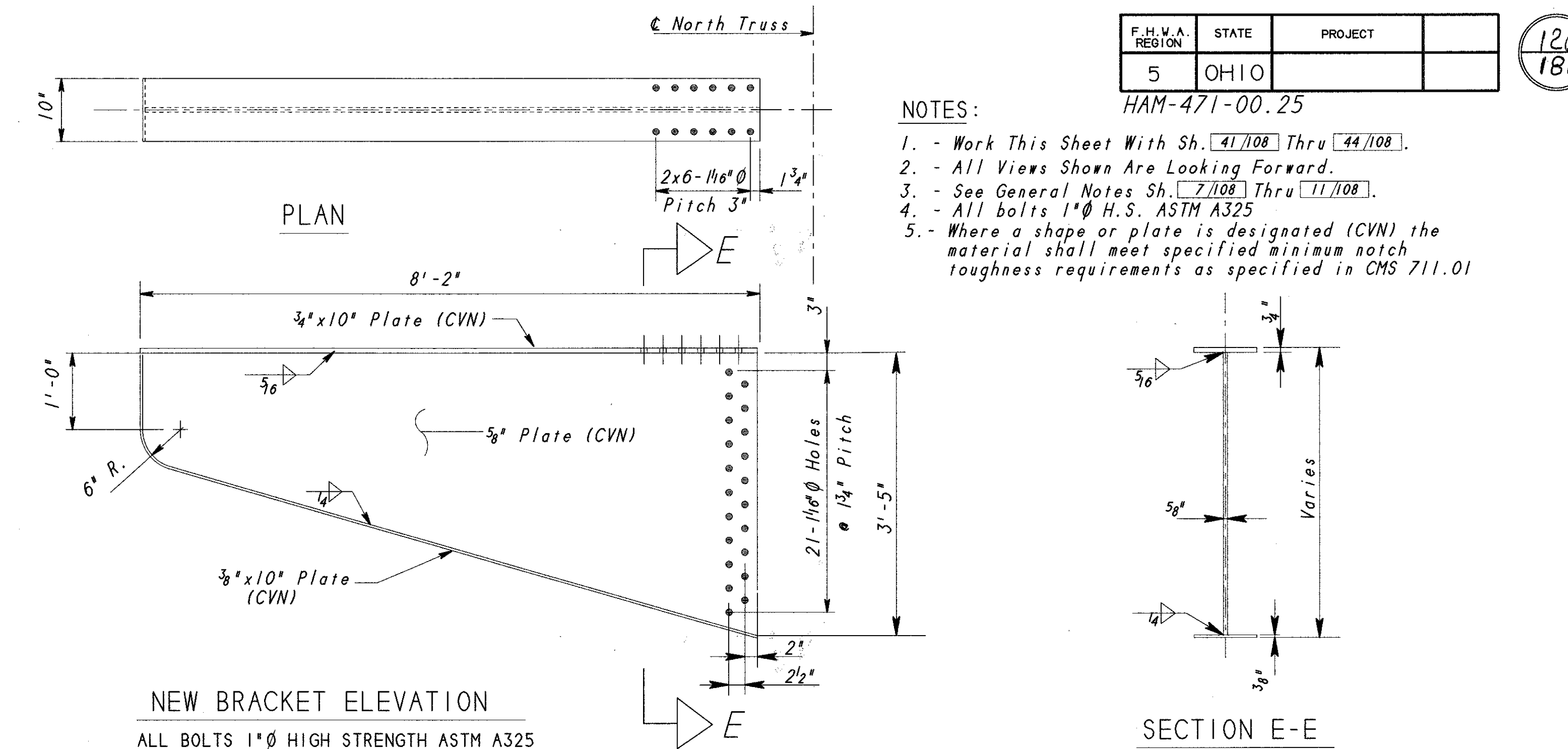
LEGEND

- Existing Structure
- New Work
- Existing hole
- New Hole Or Reamed Hole.

EXISTING CONDITIONS

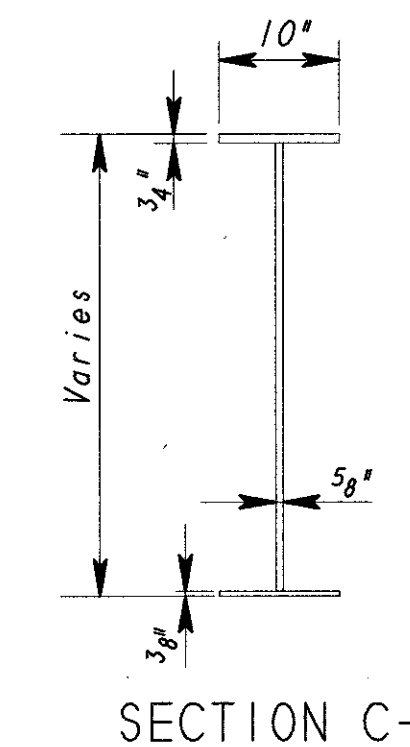
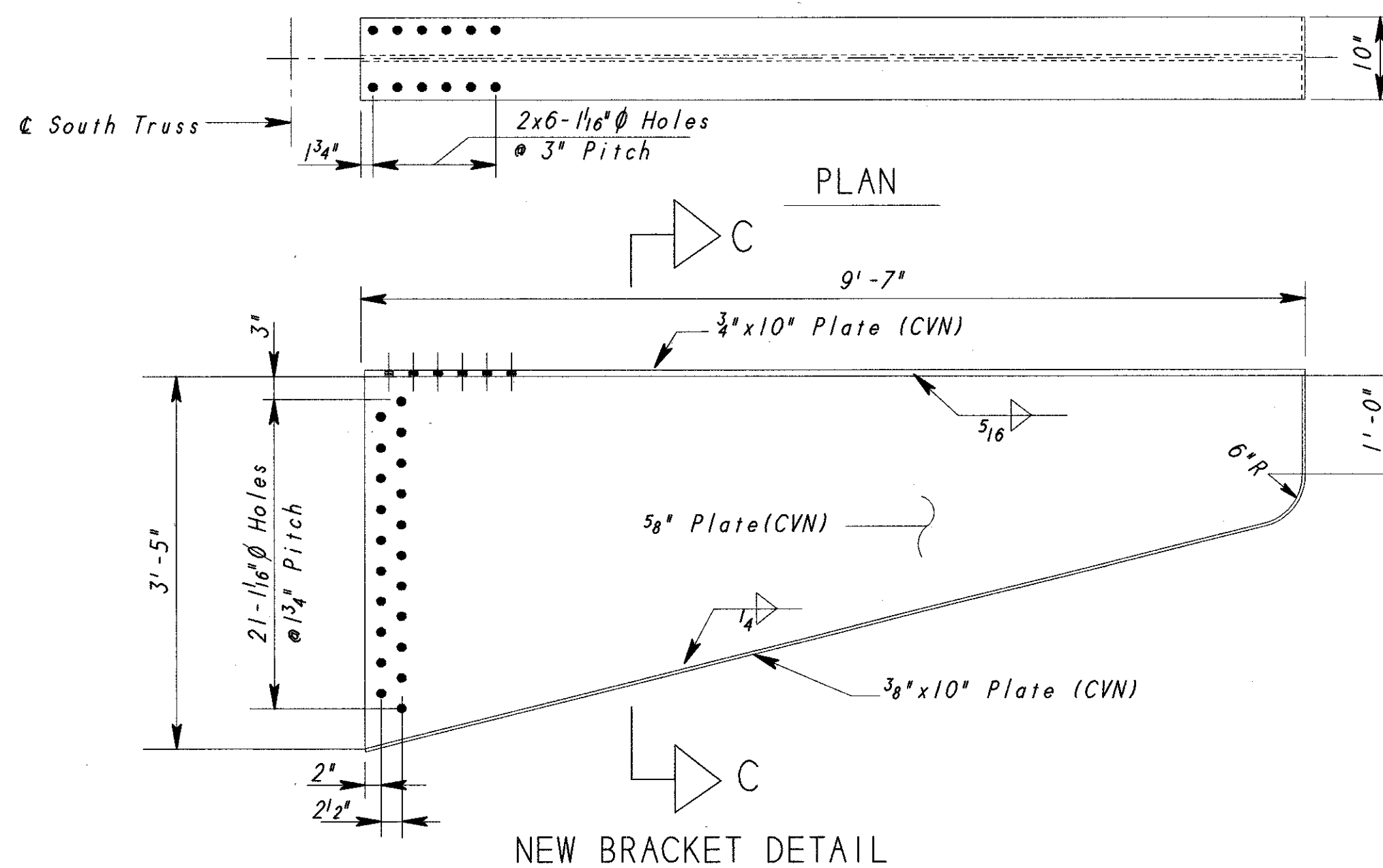
See NOTE "A" on Sh. 45/108

EXISTING BRACKET & TRUSS TOP CHORD CONNECTION
 (Representation Of Existing Structure Is Approximate)



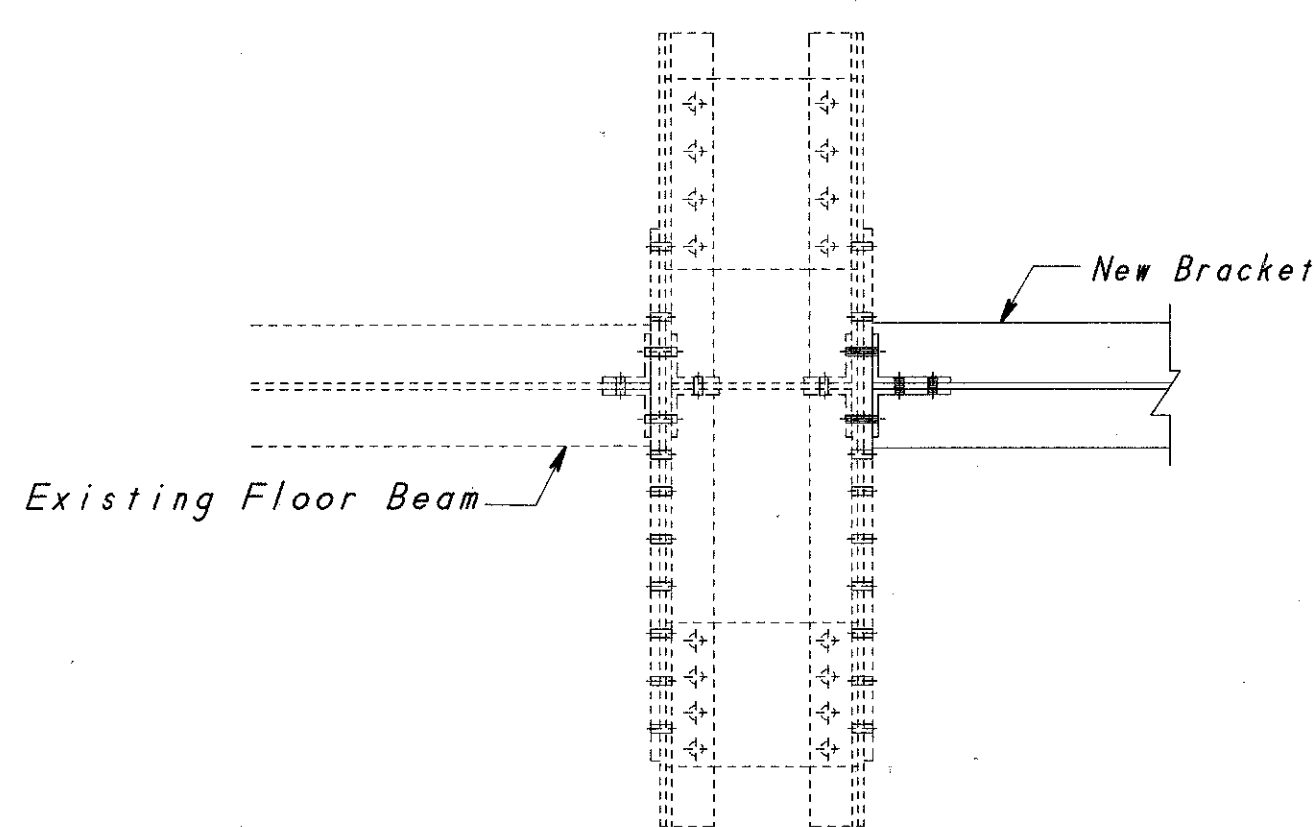
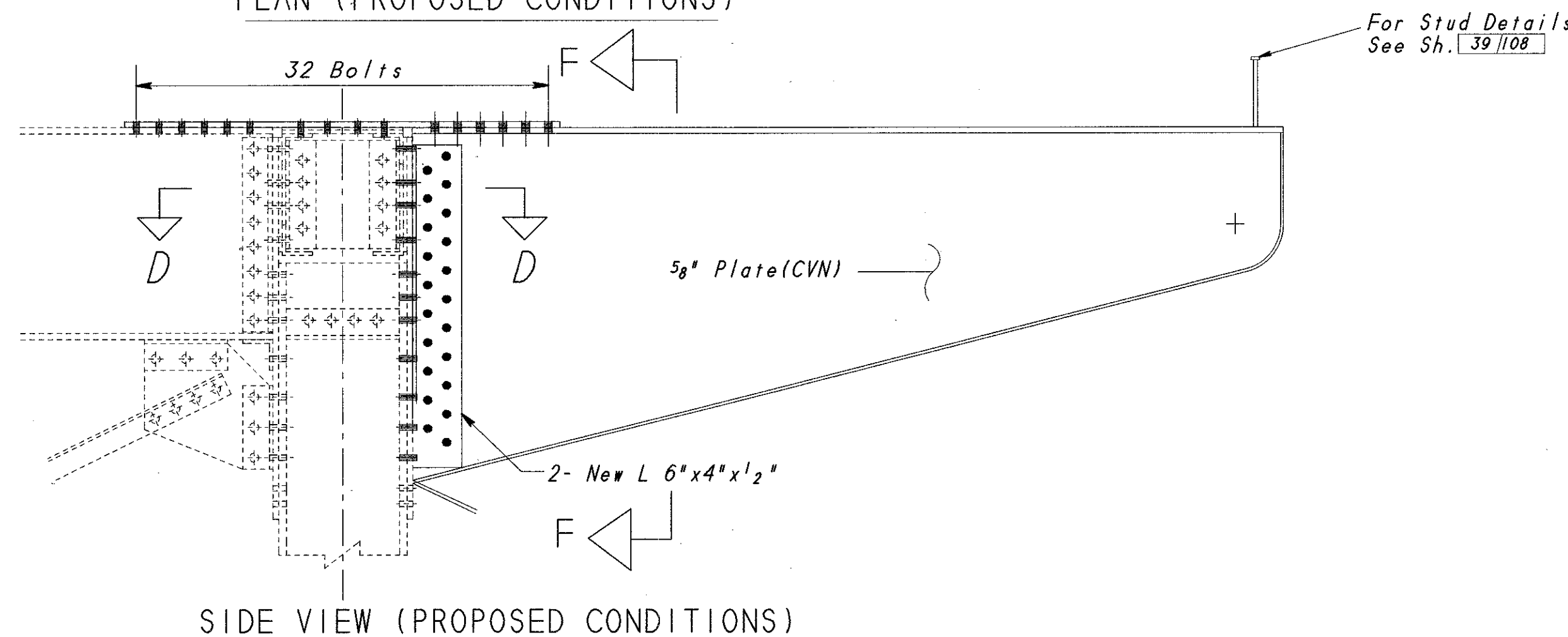
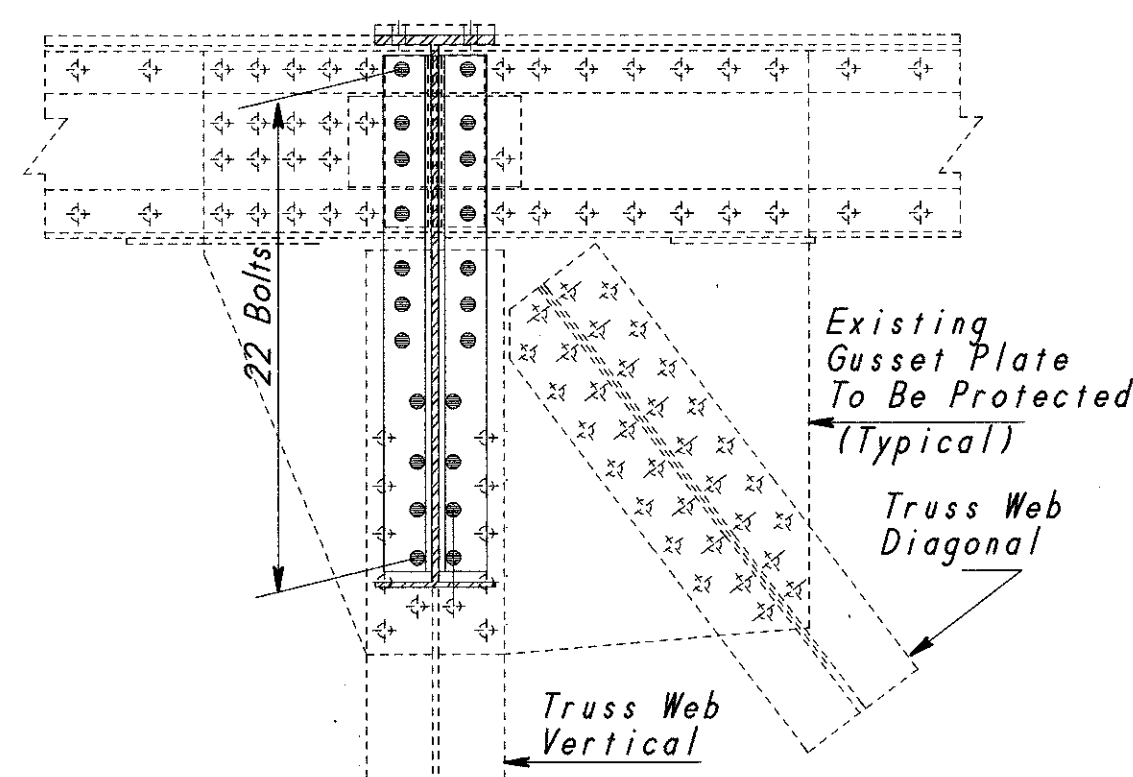
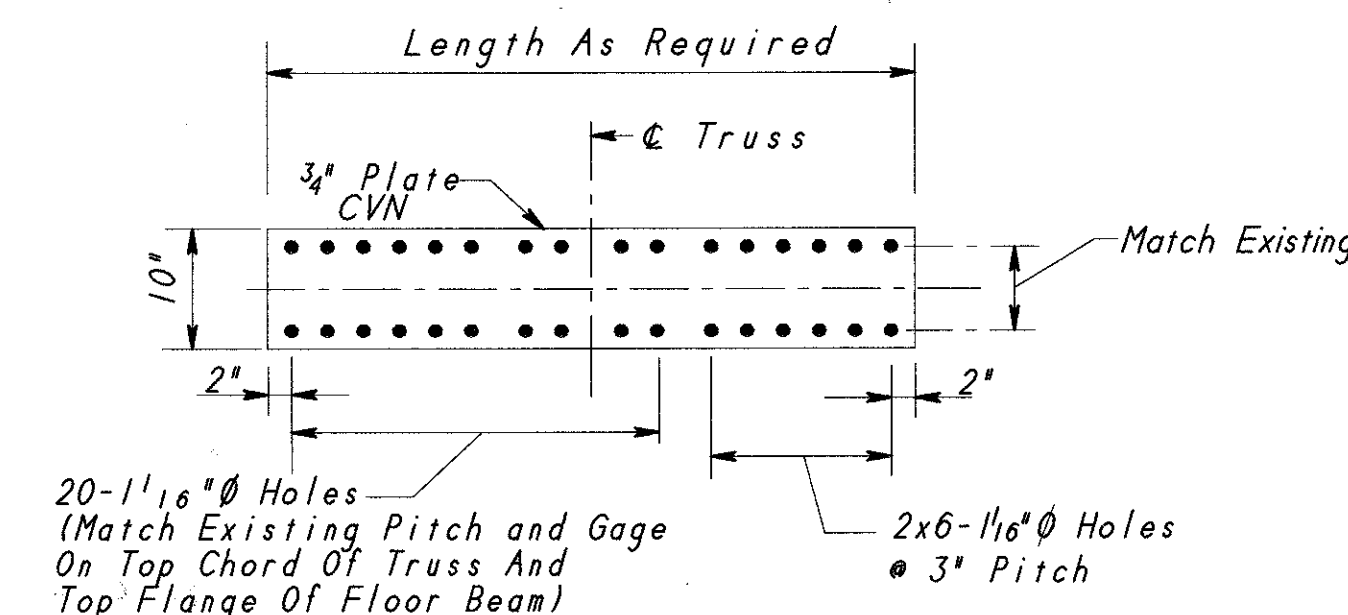
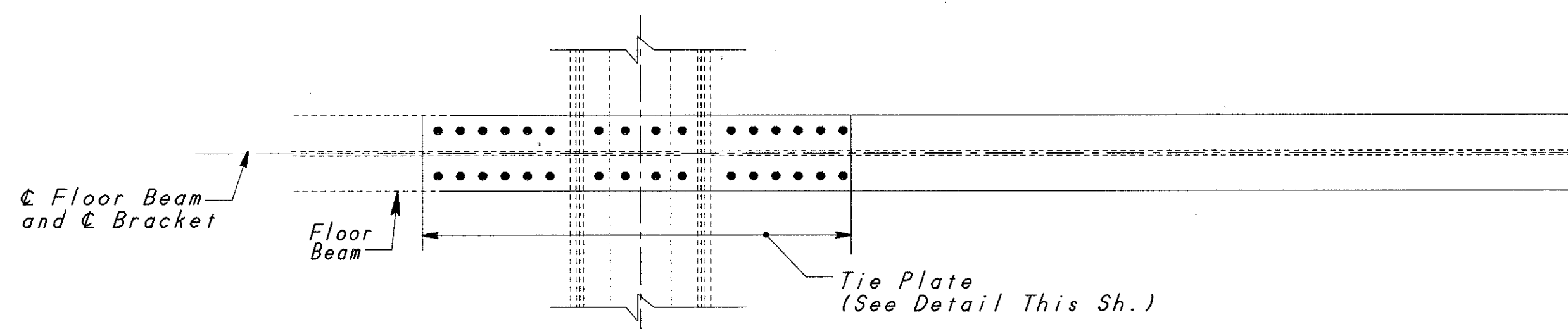
Pflum, Klausmeier & Gehrum Consultants		46/108
STRUCTURAL STEEL DETAILS BRACKETS TRUSS SPANS 5-6-7, 7-8-9-10 & 10-11-12 (UNITS E, F, & G LEFT SIDE) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY	Sta. 30+55.40	Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

BRACKET Scale 1/32



NOTES:

1. - Work This Sheet With Sh. 41/108 Thru 44/108, and Sh. 46/108
2. - Side Views Shown Are Looking Forward.
3. - See General Notes Sh. 7/108 Thru 11/108.
4. - All bolts to be 1" High Strength ASTM A325
5. - Representation Of Existing Structure Is Approximate.
6. - Where a shape or plate is designated (CVN) the material shall meet specified minimum notch toughness requirements as specified in CMS 711.01
7. - See Note "A" on Sh. 45/108



LEGEND

- Existing Structure
- New Work
- Existing hole
- New Hole Or Reamed Hole.

Pflum, Klausmeier & Gehrum Consultants		47/108
STRUCTURAL STEEL DETAILS BRACKETS		
TRUSS SPANS 5-6-7, 7-8-9-10 & 10-11-12		
(UNITS E, F, & G RIGHT SIDE)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/JDR	CHECKED WDD
TRACED	REVIEWED IEH April 96	DATE APR 96
		REVISED

NOTES -

1. - Downspout installation depicted on this sheet will not require shop drawings prior to fabrication. The Contractor shall remove the existing downspout including clamps, shall burn anchor bolts down to a minimum of 1" below the concrete surface, and seal hole with epoxy mortar. The Contractor shall make the necessary field measurements, and prepare sketches, drawings, tables, etc. The Contractor shall field verify the new downspout location and run, and include this information with the shop drawings. After fabrication and installation, the Contractor shall submit shop drawings to the Engineer for review and approval. The Engineer shall have authority and responsibility for ensuring that the fabricated steel is acceptable.

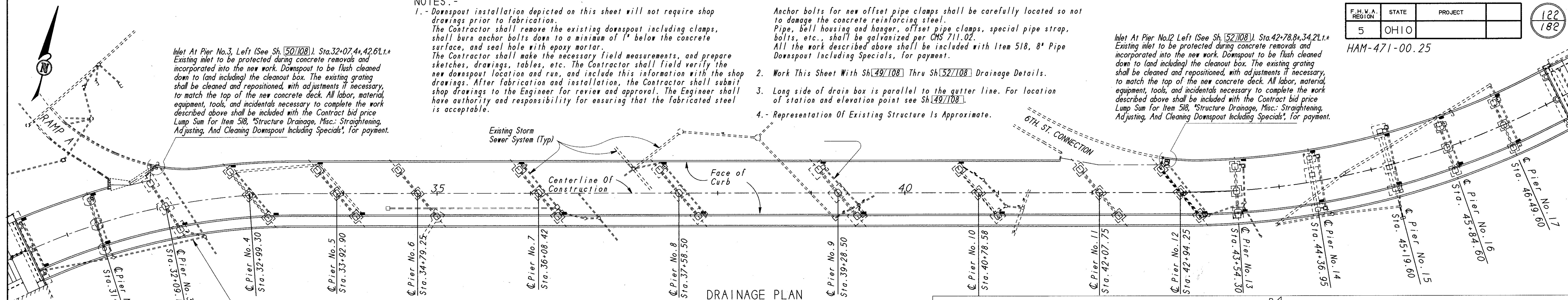
Anchor bolts for new offset pipe clamps shall be carefully located so not to damage the concrete reinforcing steel. Pipe, bell housing and hanger, offset pipe clamps, special pipe strap, bolts, etc., shall be galvanized per CMS 711.02. All the work described above shall be included with Item 518, 8" Pipe Downspout Including Specials, for payment.

Inlet At Pier No.12 Left (See Sh. 52108). Sta.42+78.8+34.2L+ Existing inlet to be protected during concrete removals and incorporated into the new work. Downspout to be flush cleaned down to land including the cleanout box. The existing grating shall be cleaned and repositioned, with adjustments if necessary, to match the top of the new concrete deck. All labor, material, equipment, tools, and incidentals necessary to complete the work described above shall be included with the Contract bid price Lump Sum for Item 518, "Structure Drainage, Misc.: Straightening, Adjusting, And Cleaning Downspout Including Specials", for payment.

2. Work This Sheet With Sh.497108 Thru Sh.527108 Drainage Details.
3. Long side of drain box is parallel to the gutter line. For location of station and elevation point see Sh.497108.
4. Representation Of Existing Structure Is Approximate.

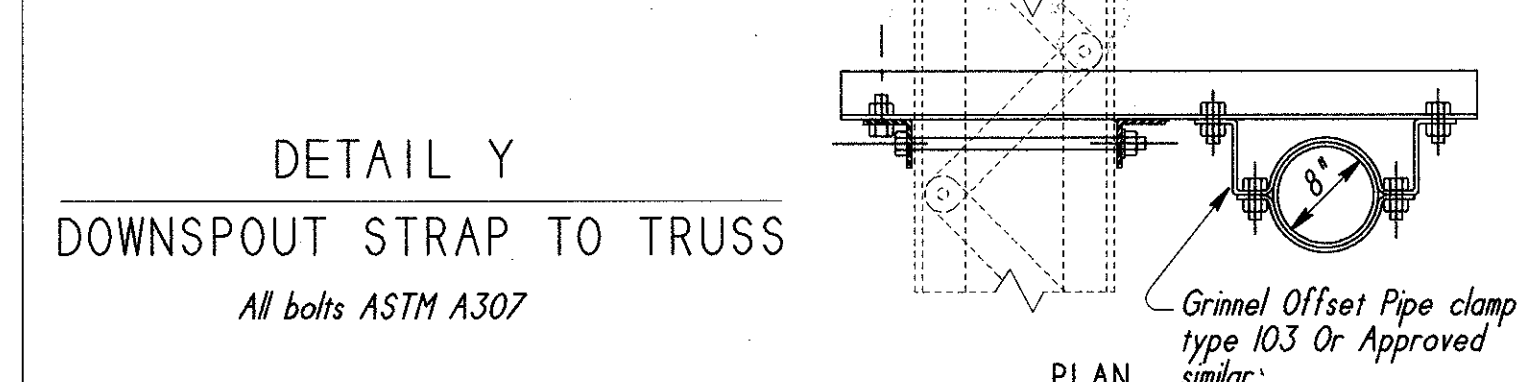
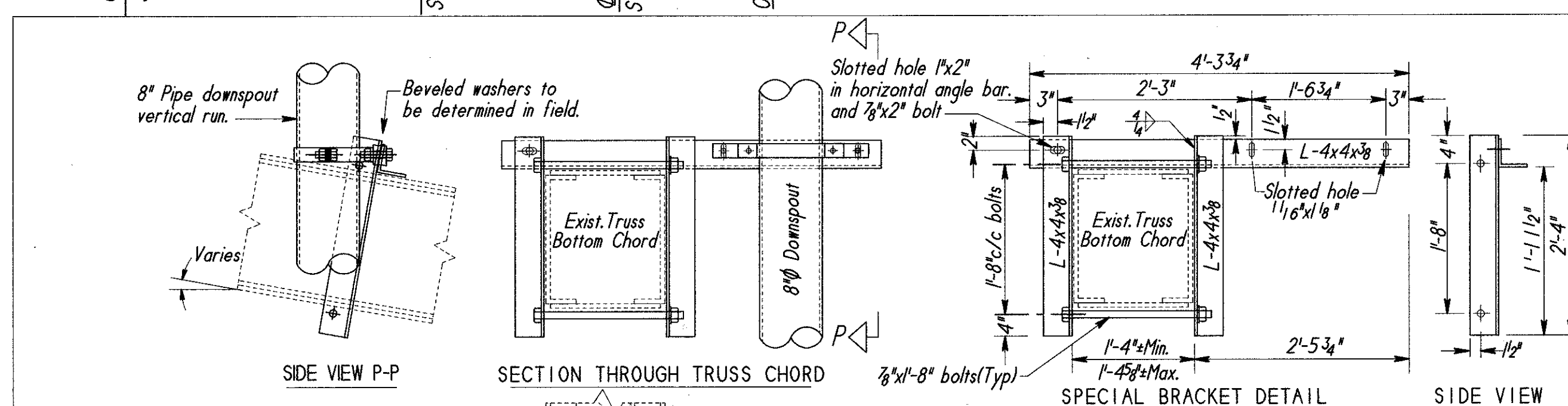
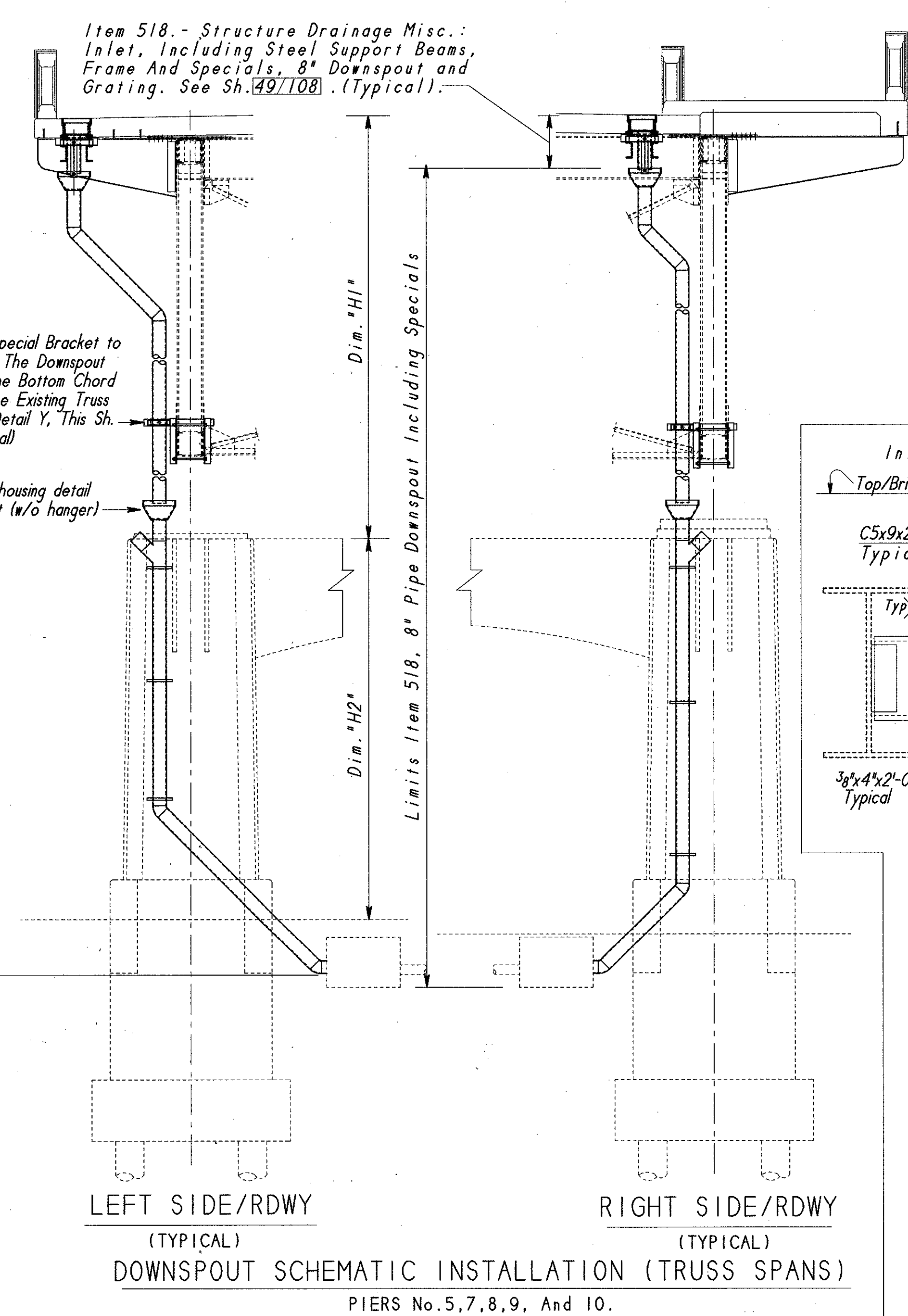
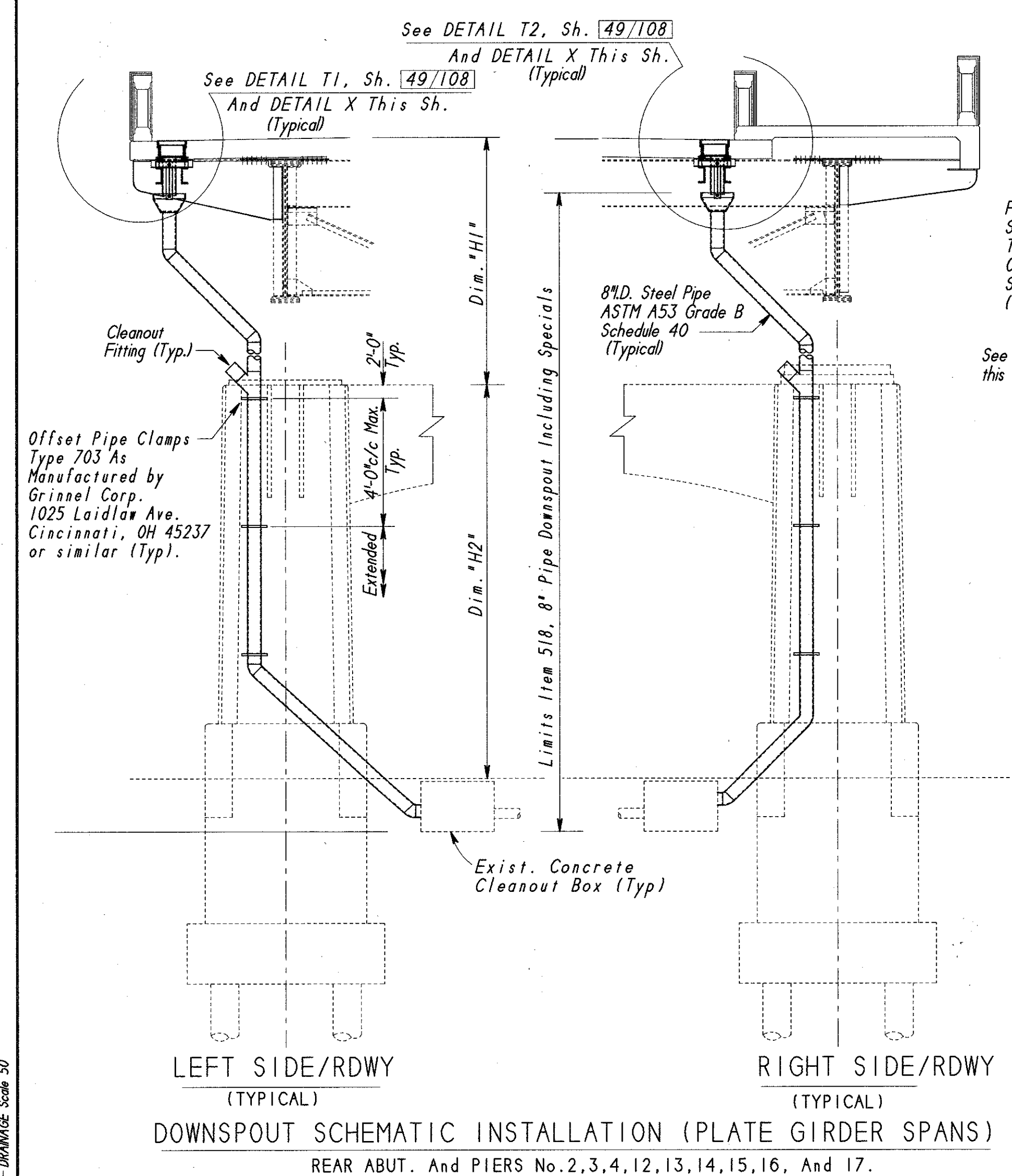
Inlet At Pier No.3, Left (See Sh. 507108). Sta.32+07.4+42.6L+ Existing inlet to be protected during concrete removals and incorporated into the new work. Downspout to be flush cleaned down to land including the cleanout box. The existing grating shall be cleaned and repositioned, with adjustments if necessary, to match the top of the new concrete deck. All labor, material, equipment, tools, and incidentals necessary to complete the work described above shall be included with the Contract bid price Lump Sum for Item 518, "Structure Drainage, Misc.: Straightening, Adjusting, And Cleaning Downspout Including Specials", for payment.

All in-ground cleanout boxes, and connecting pipes shall be flush cleaned to the nearest municipal manhole. All labor, equipment, tools, and incidentals necessary to complete this work shall be included with the Contract bid price Lump Sum for Item 518, "Structure Drainage, Misc.: Straightening, Adjusting, And Cleaning Downspout Including Specials", for payment.



LEGEND

- Existing Drain Inlet To Be Incorporated Into The New Work.
- New Drain Inlet



DRAINAGE INLET SCHEDULE					
Location	Position Of Inlet Station, Offset	Final Elev. At Top/Frame	Dim. H1'	Dim. H2'	
Rear Abutment	30+59.78, 33.88 Lt	538.13	10.9'±	9.2'±	
	30+61.75, 22.65 Rt	538.16	11'±	9.2'±	
	31+37.11, 33.02 Lt	539.00	11'±	13.5'±	
No. 2	31+37.42, 22.65 Rt	539.01	11'±	13.5'±	
No. 3	Left Side N/A				
	32+23.18, 22.65 Rt	540.43	12.3'±	16.1'±	
No. 4	32+84.68, 33.06 Lt	541.62	11.7'±	20.9'±	
	33+21.32, 22.65 Rt	542.43	12.5'±	20.9'±	
No. 5	33+75.72, 33.06 Lt	543.81	14.3'±	21.5'±	
	34+18.22, 22.65 Rt	545.03	15.5'±	21.5'±	
No. 7	35+91.22, 33.06 Lt	550.69	24.5'±	24.2'±	
	36+33.71, 22.65 Rt	552.10	25.8'±	24.3'±	
No. 8	37+39.96, 33.06 Lt	555.62	31.2'±	25.4'±	
	37+82.46, 22.65 Rt	557.02	32.5'±	25.5'±	
No. 9	39+09.96, 33.06 Lt	561.24	30.3'±	37.9'±	
	39+52.46, 22.65 Rt	562.65	31.8'±	37.9'±	
No. 10	40+61.71, 33.06 Lt	566.27	24.5'±	29.3'±	
	41+04.21, 22.65 Rt	567.67	26'±	29.2'±	
No. 12	Left Side N/A				
	43+20.75, 22.65 Rt	574.84	15.8'±	28.0'±	
No. 13	43+58.51, 33.06 Lt	576.09	11'±	31.1'±	
	43+58.24, 22.65 Rt	576.08	11'±	31.1'±	
No. 14	44+40.96, 33.06 Lt	578.82	11'±	27.8'±	
	44+40.71, 22.65 Rt	578.81	11'±	27.8'±	
No. 15	45+23.94, 33.06 Lt	581.56	11'±	22.6'±	
	45+23.69, 22.65 Rt	581.56	11'±	22.6'±	
No. 16	45+88.79, 33.06 Lt	583.71	10'±	20.7'±	
	45+88.56, 22.65 Rt	583.70	10'±	20.7'±	
No. 17	46+53.71, 33.06 Lt	585.86	10'±	18.9'±	
	46+53.39, 22.65 Rt	585.85	10'±	18.9'±	

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Consultants
48 / 108

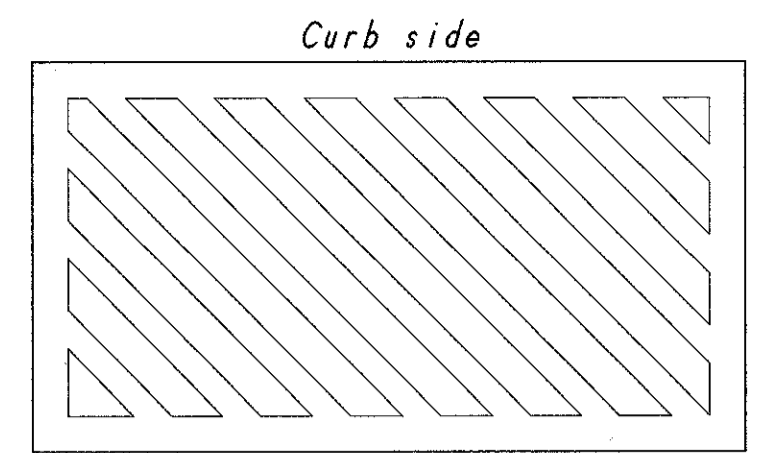
DECK DRAINAGE PLAN AND DOWNSPOUT DETAILS

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

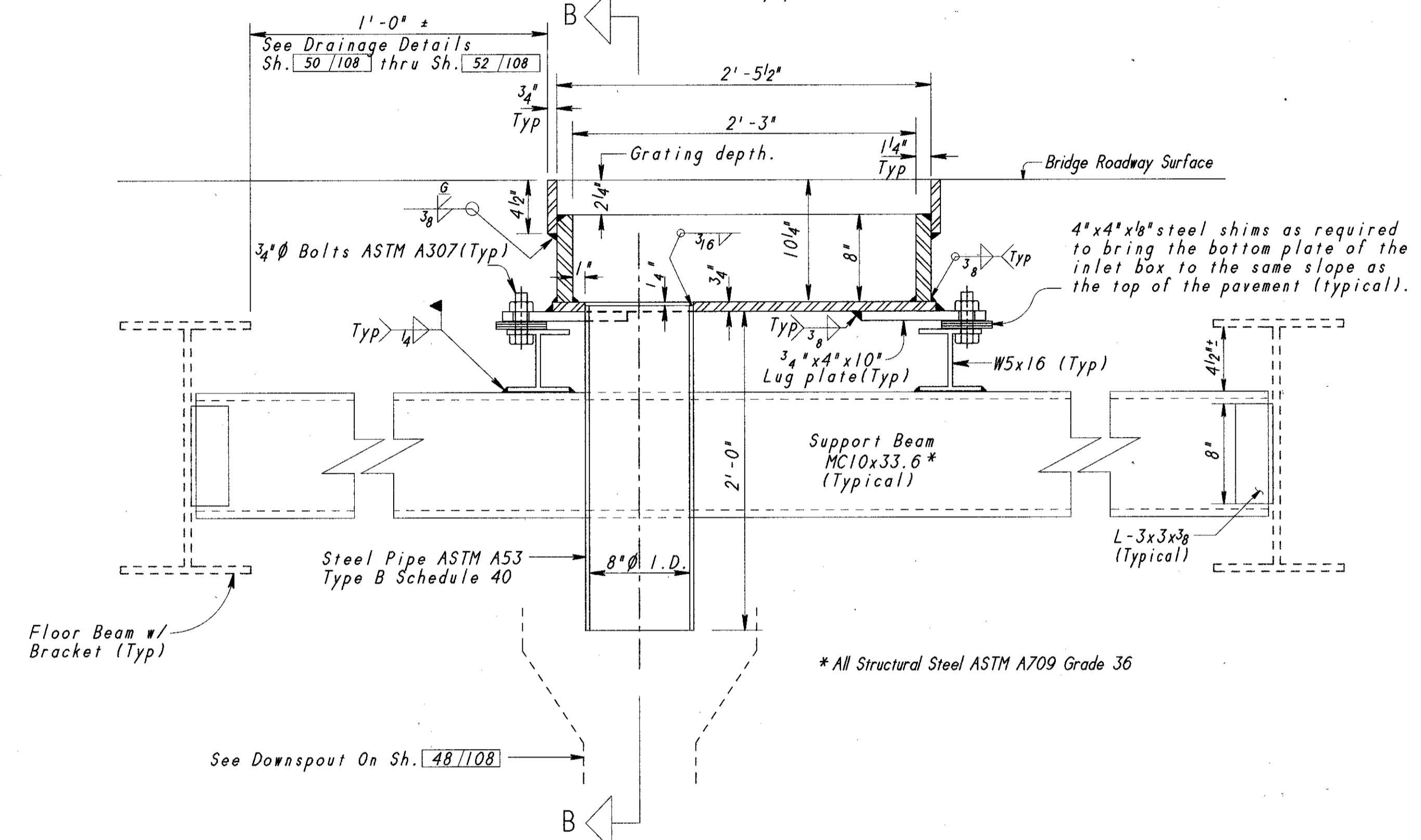
HAMILTON COUNTY
Sta. 30+55.40
Sta. 47+16.19

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED/JDR		WDD	IEH	April 96	

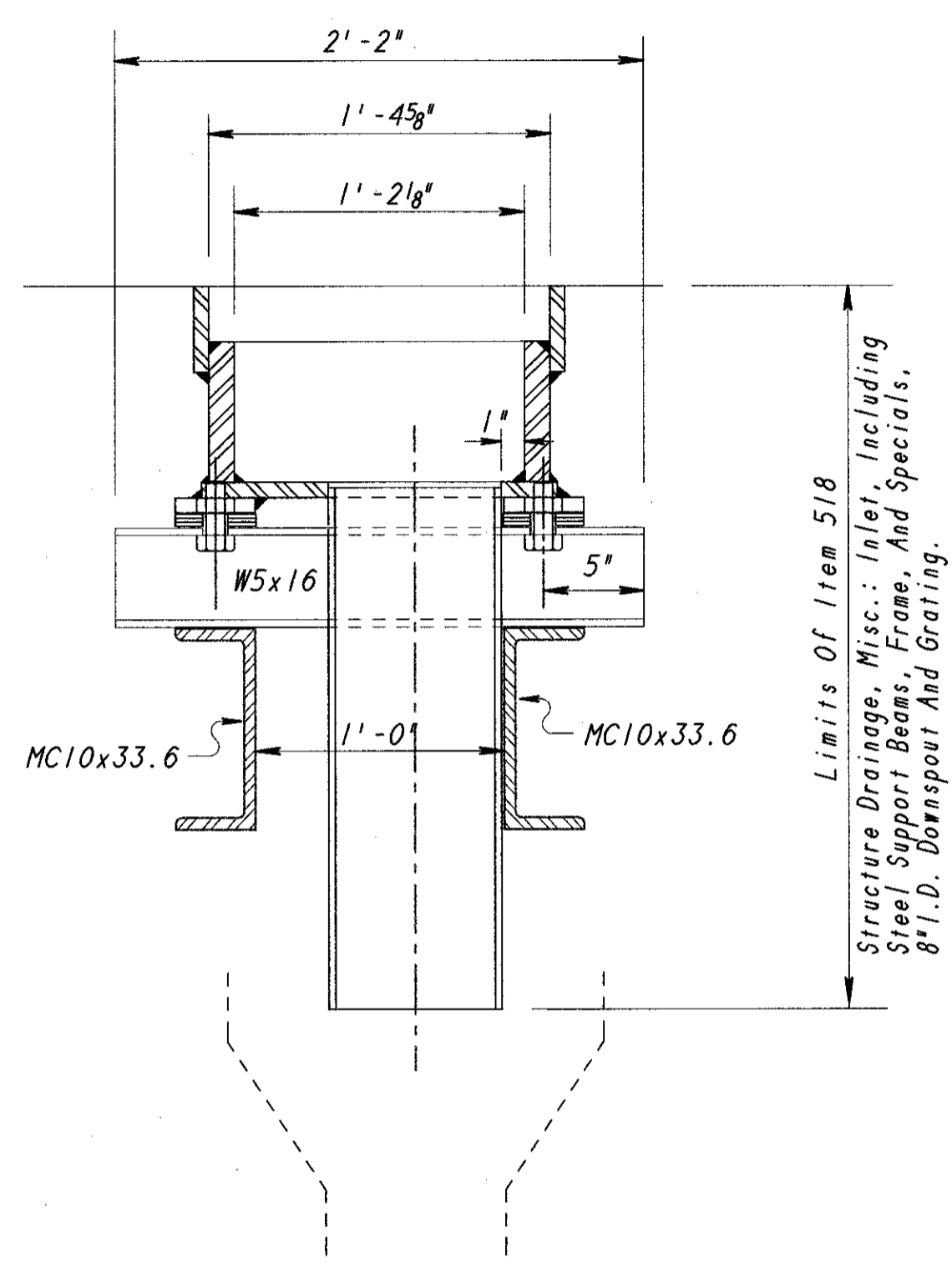
DRAWING Scale 50



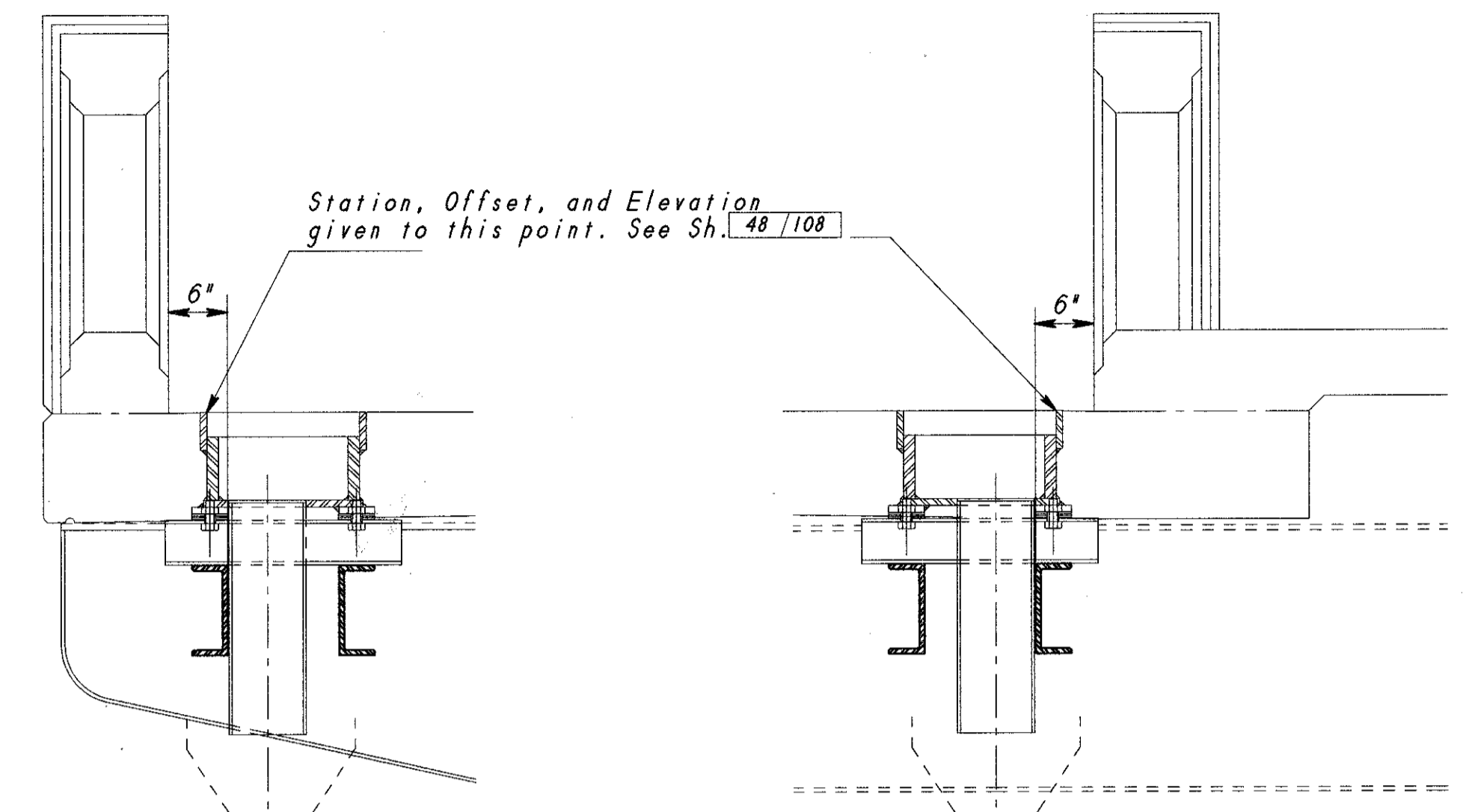
GRATING PLAN
(See roadway plans)



SECTION A-A
(GRATING NOT SHOWN)

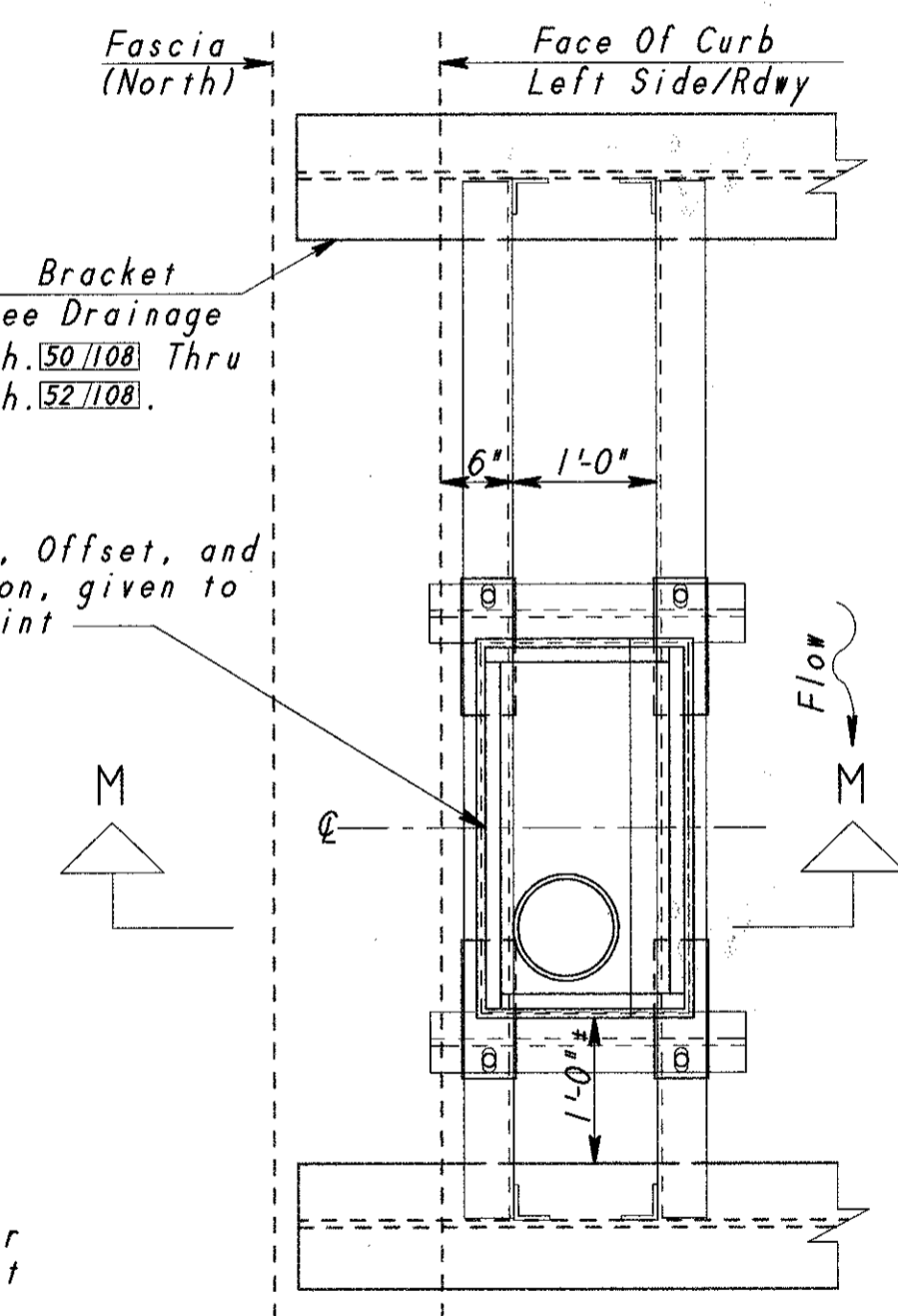


SECTION B-B
(GRATING NOT SHOWN)

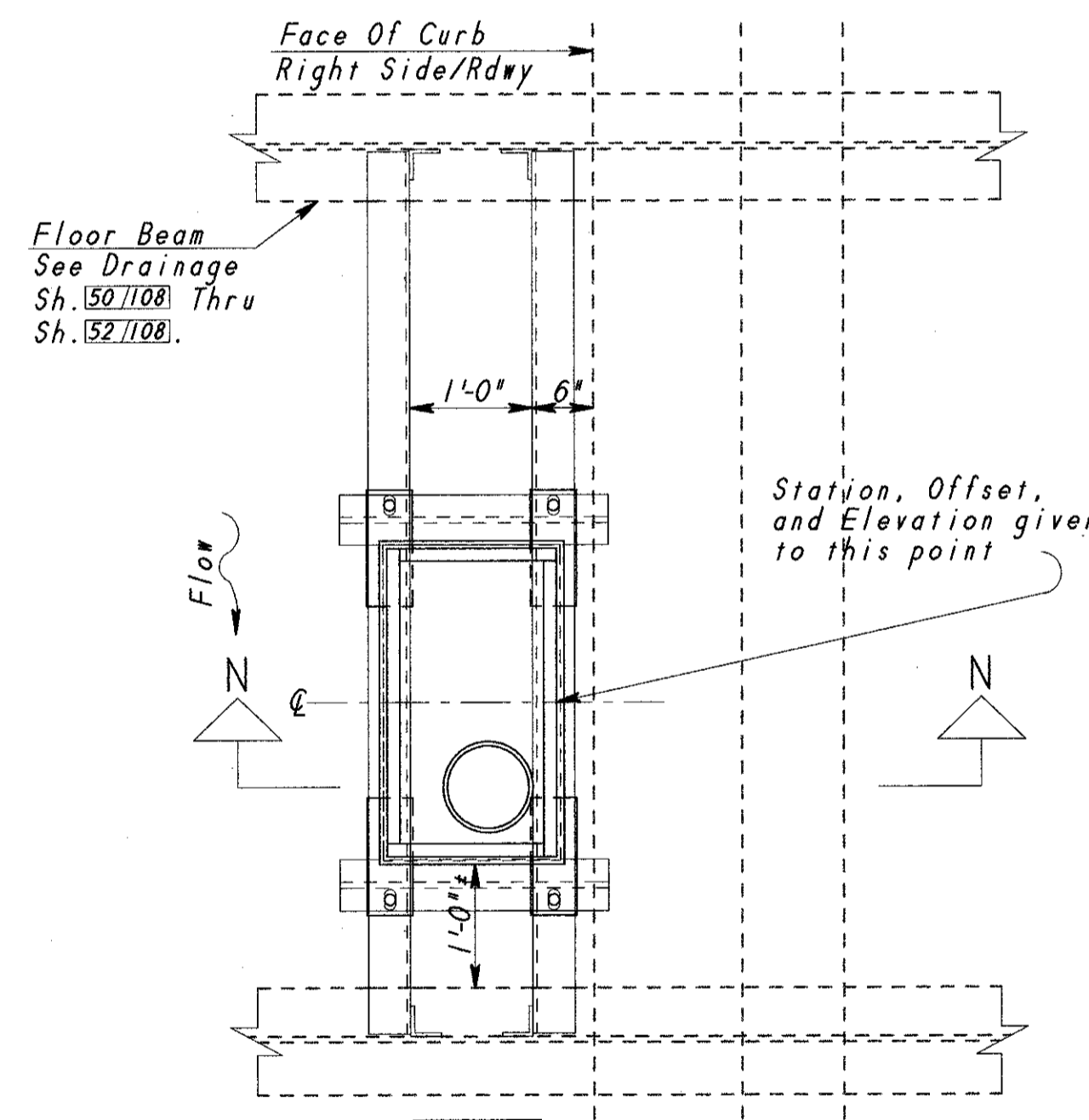


SECTION M-M

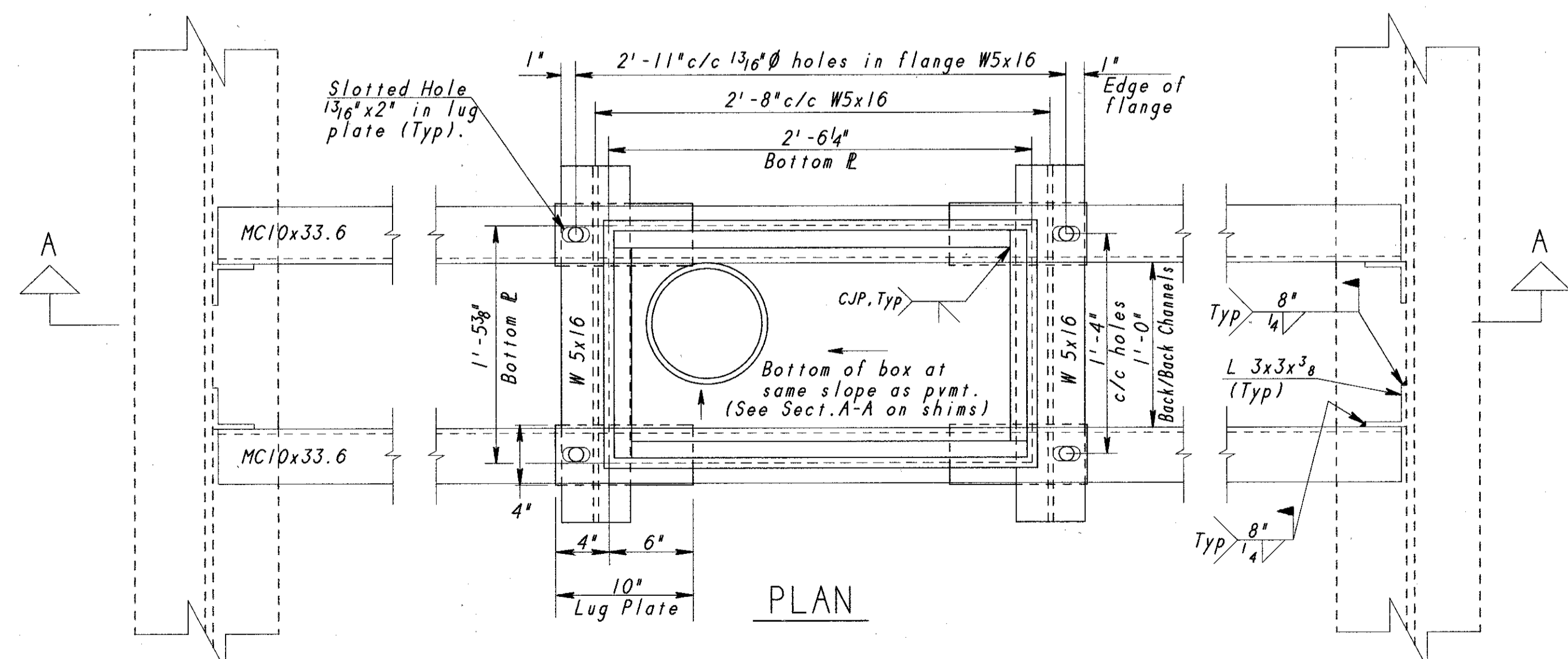
SECTION N-N



PLAN
DETAIL T1



PLAN
DETAIL T2



TYPICAL INLET
(AND INLET INSTALLATION)

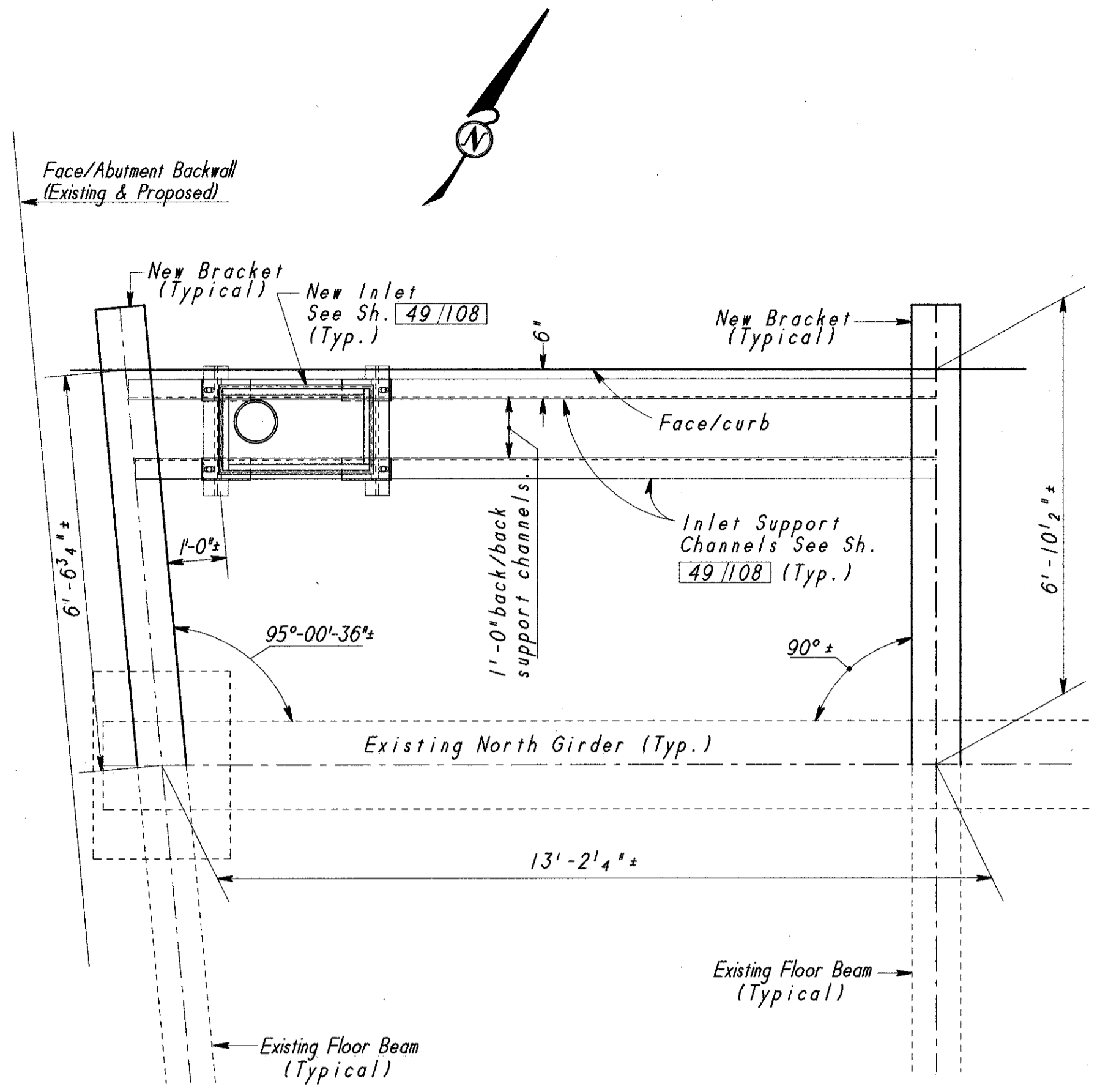
NOTE -
The typical drainage inlet described on this sheet will not require shop drawings prior to fabrication. The Contractor shall make the necessary field measurements, and prepare sketches, drawings, tables, etc. The Engineer shall have authority and responsibility for ensuring that the fabricated steel is acceptable. The Contractor shall field verify the inlet location, and include this information with the shop drawings. After fabrication and installation the Contractor shall submit shop drawings to the Engineer for review, and approval.
The steel support beams shall be installed such that once the inlet installation is completed, the top of the inlet frame and grating will rest flush with, and on the same plane and slope as the bridge roadway surface.
Support beams, strap angles, shims, etc., shall be painted per CMS 514 System 12EU, retouched after erection. Color of paint shall be black, or as approved by the City of Cincinnati.
Inlet frame, bolts, pipe downspout, grating, etc., shall be galvanized per CMS 711.02.
All work described above shall be included with Item 518, STRUCTURE DRAINAGE, MISC.: INLET, INCLUDING STEEL SUPPORT BEAMS, FRAME, AND SPECIALS, 8" DOWNSPOUT, AND GRATING, for payment.

Work Details T1 and T2 with DRAINAGE, Sh. 48/108.
And With Drainage Details, Inlets Location, Sh. 50/108 Thru Sh. 52/108.

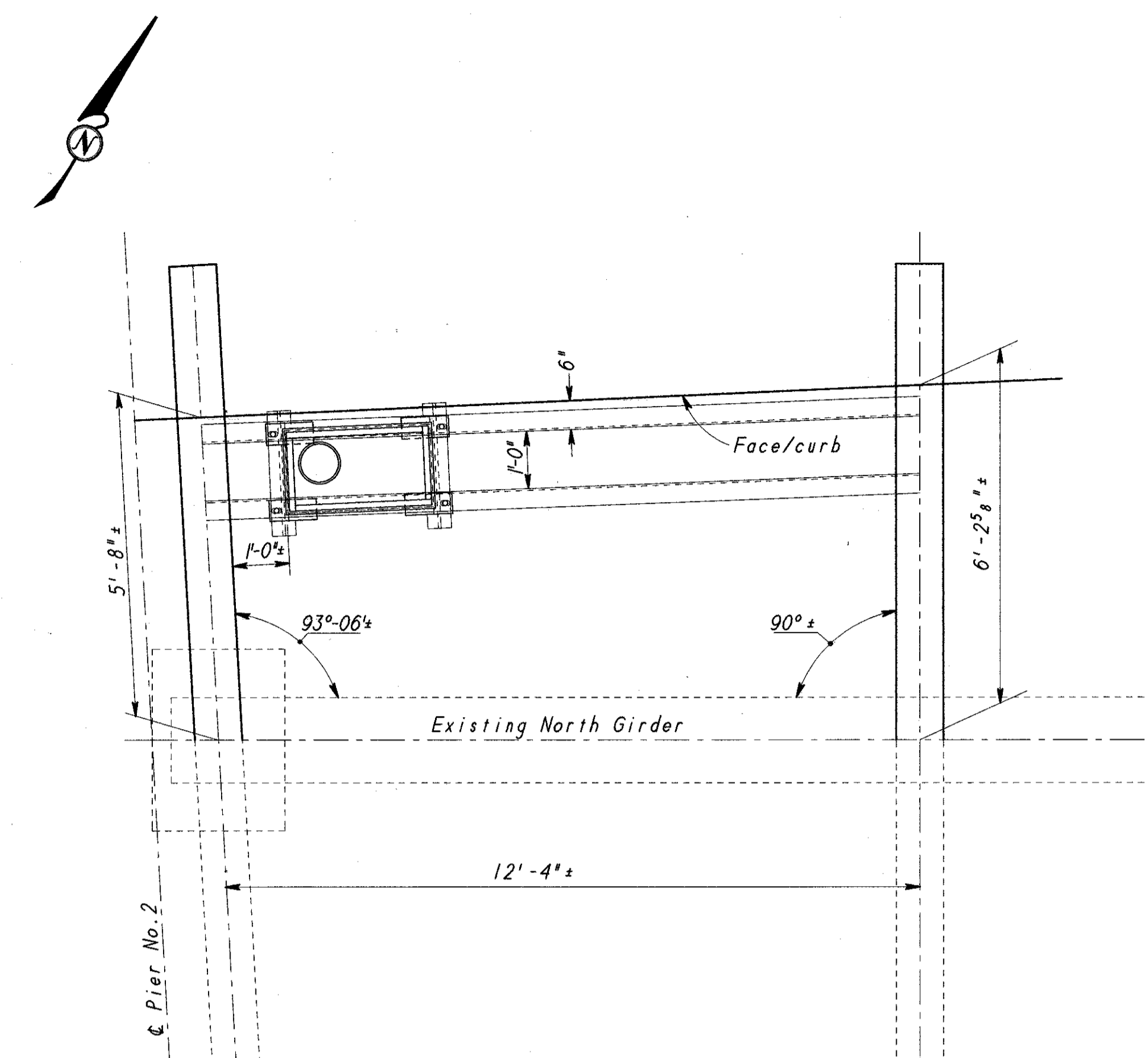
Plum, Klausmeier & Gehrm		49/108
Consultants		OHIO
DRAINAGE DETAILS TYPICAL INLET		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED/JDR	WDD
CHECKED	DATE	REVISED
WDD	1EH April 96	

DRAIN/DTI Scale .6666

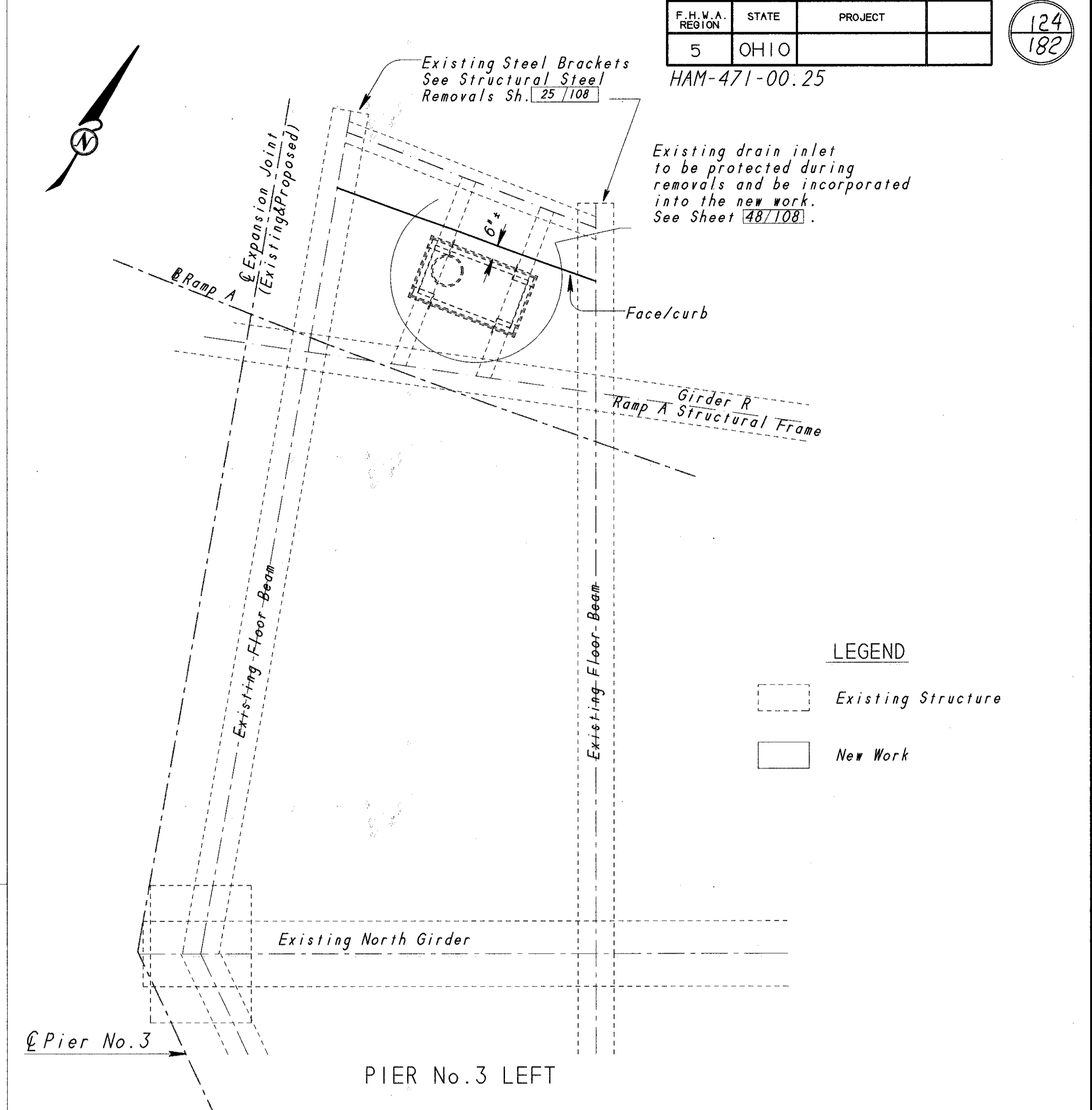
HAM-471-00.25



REAR ABUTMENT LEFT



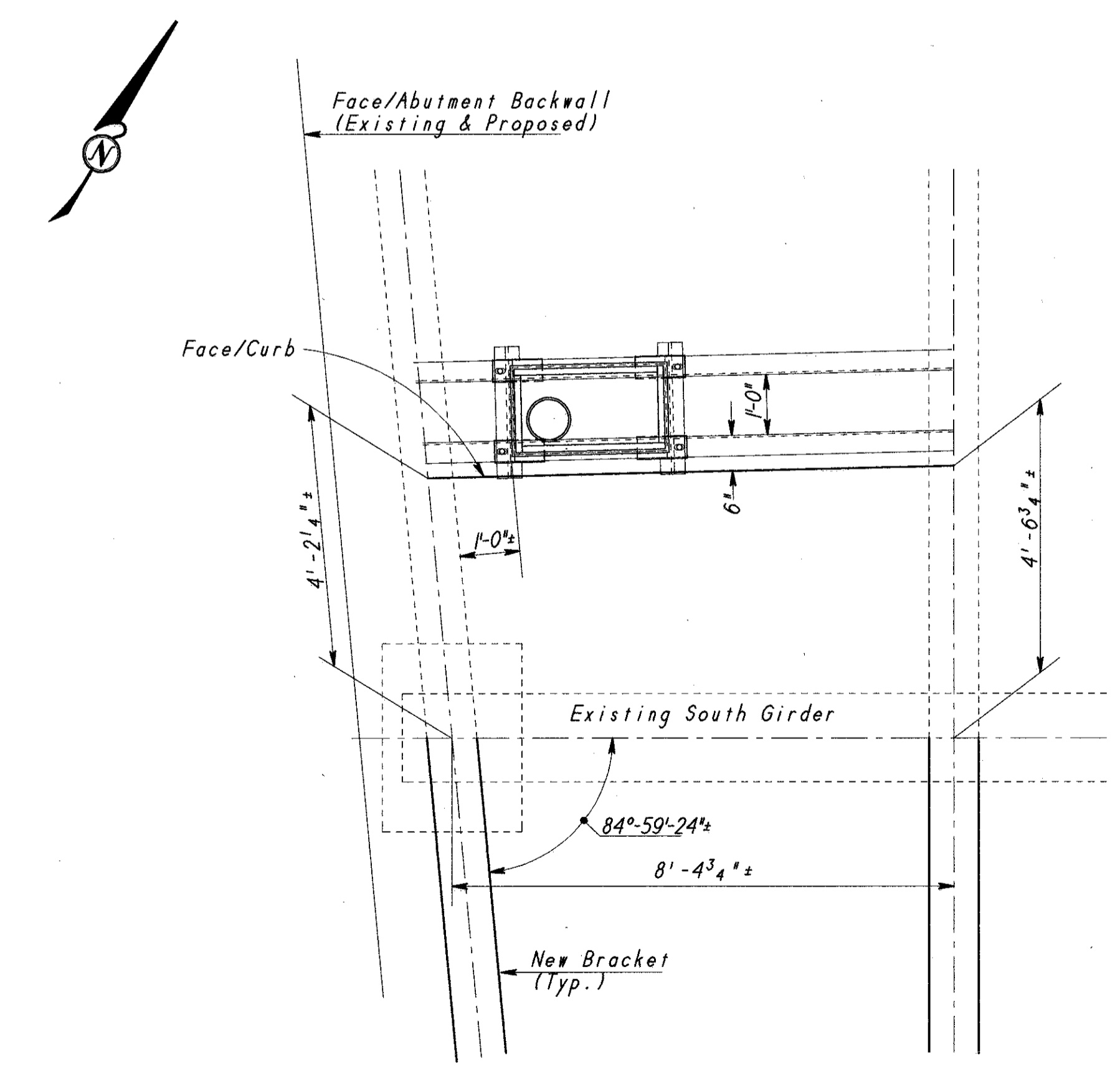
PIER No. 2 LEFT



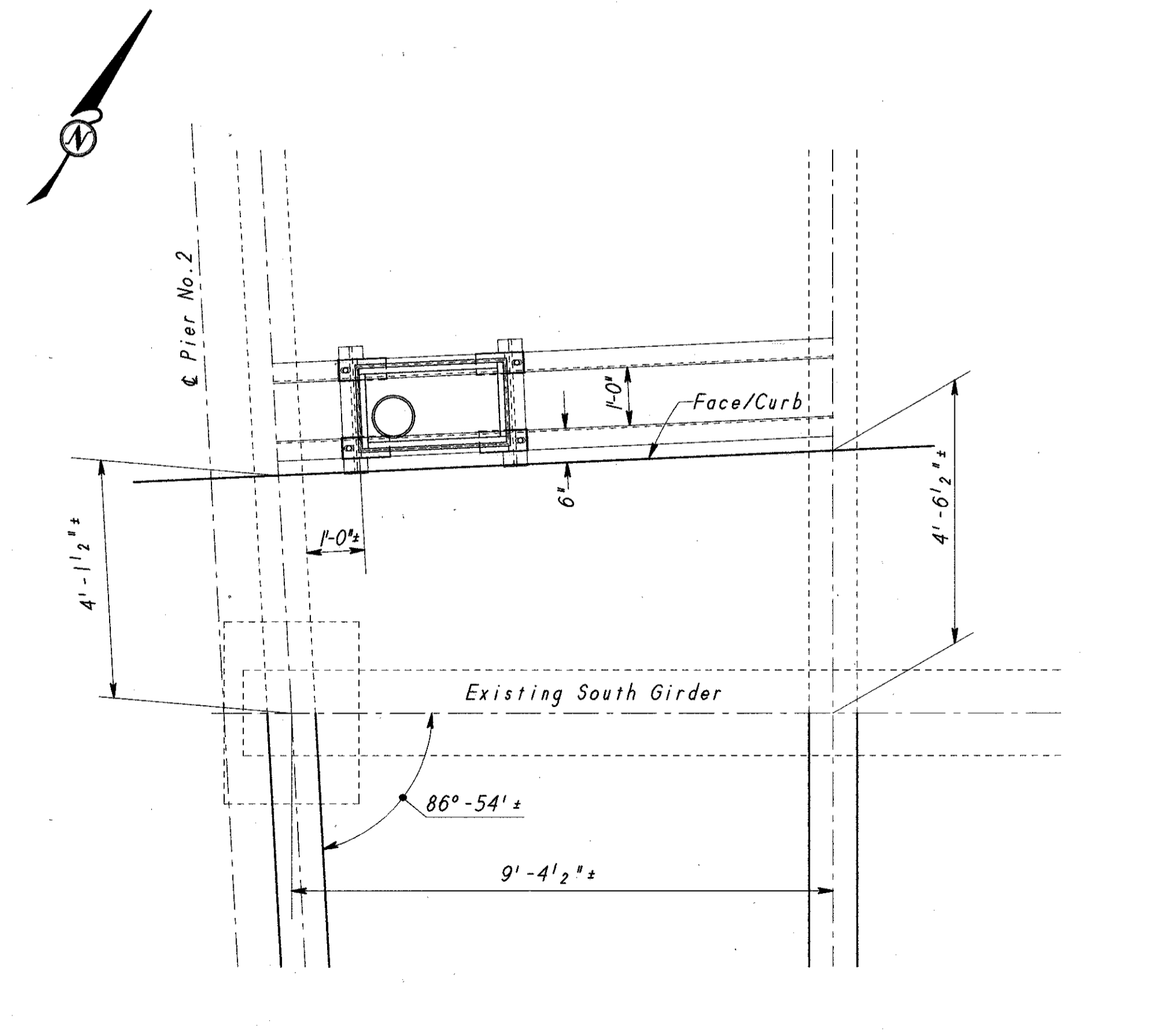
PIER No. 3 LEFT

LEGEND

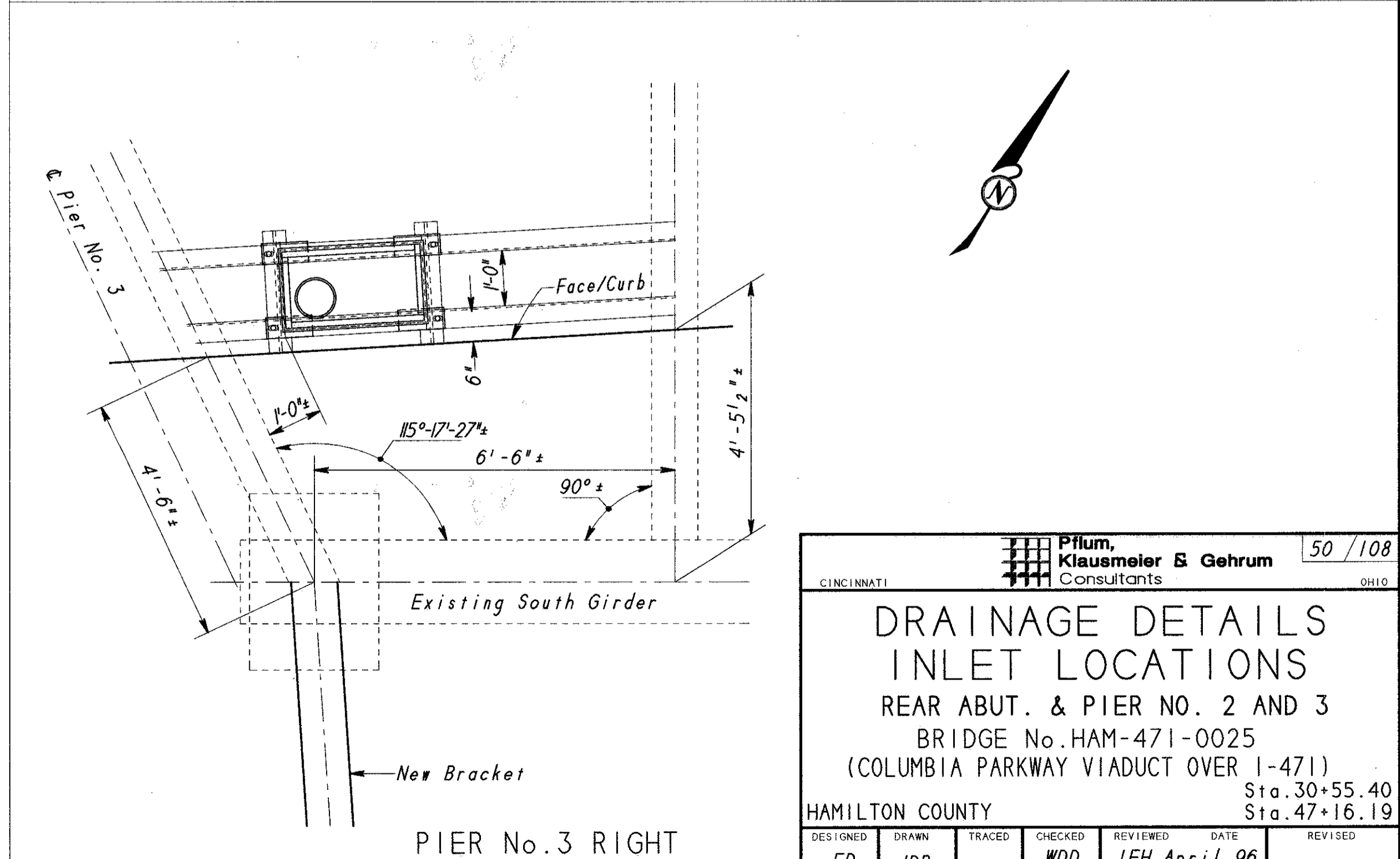
- Existing Structure
- New Work



REAR ABUTMENT RIGHT



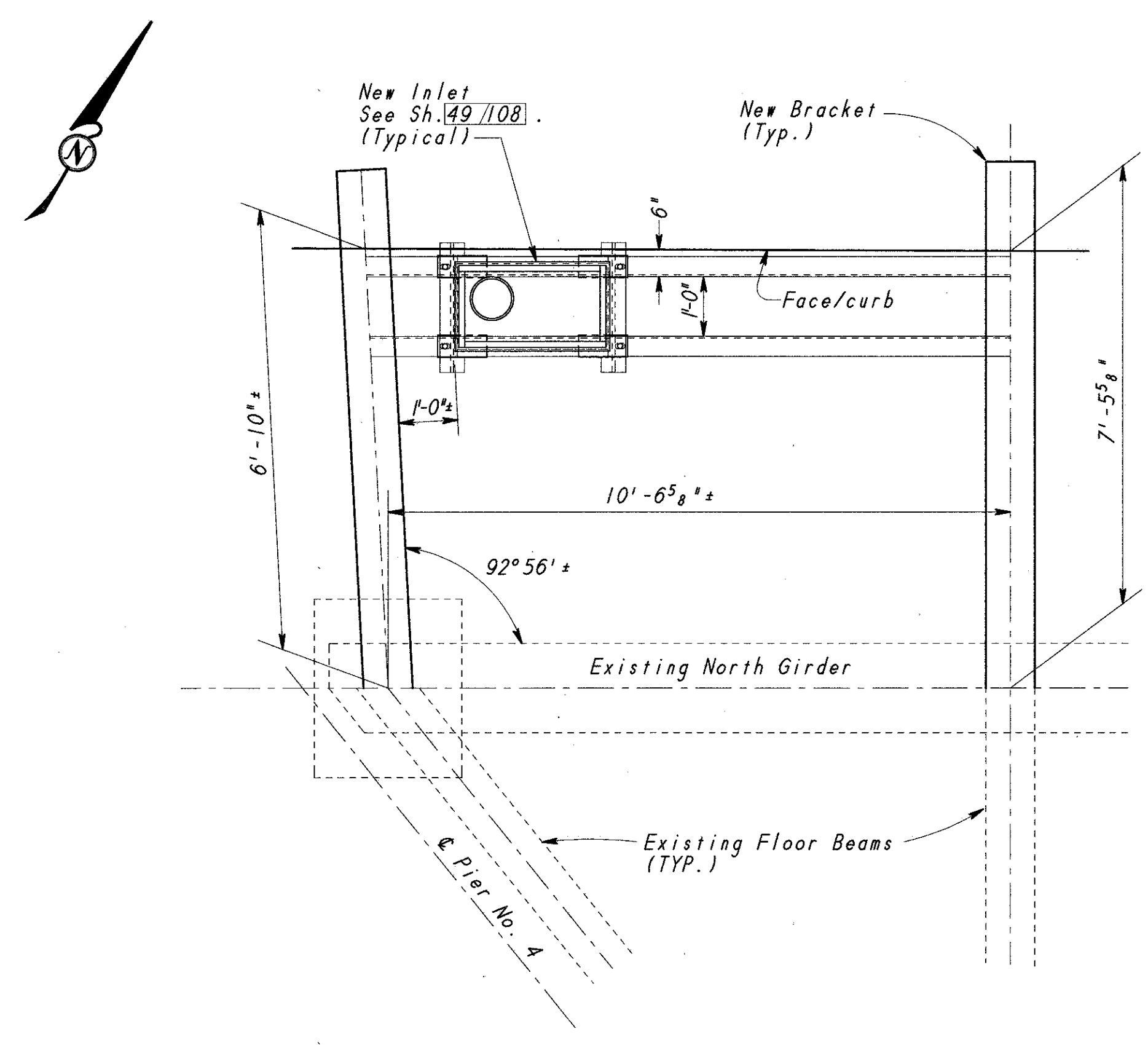
PIER No. 2 RIGHT



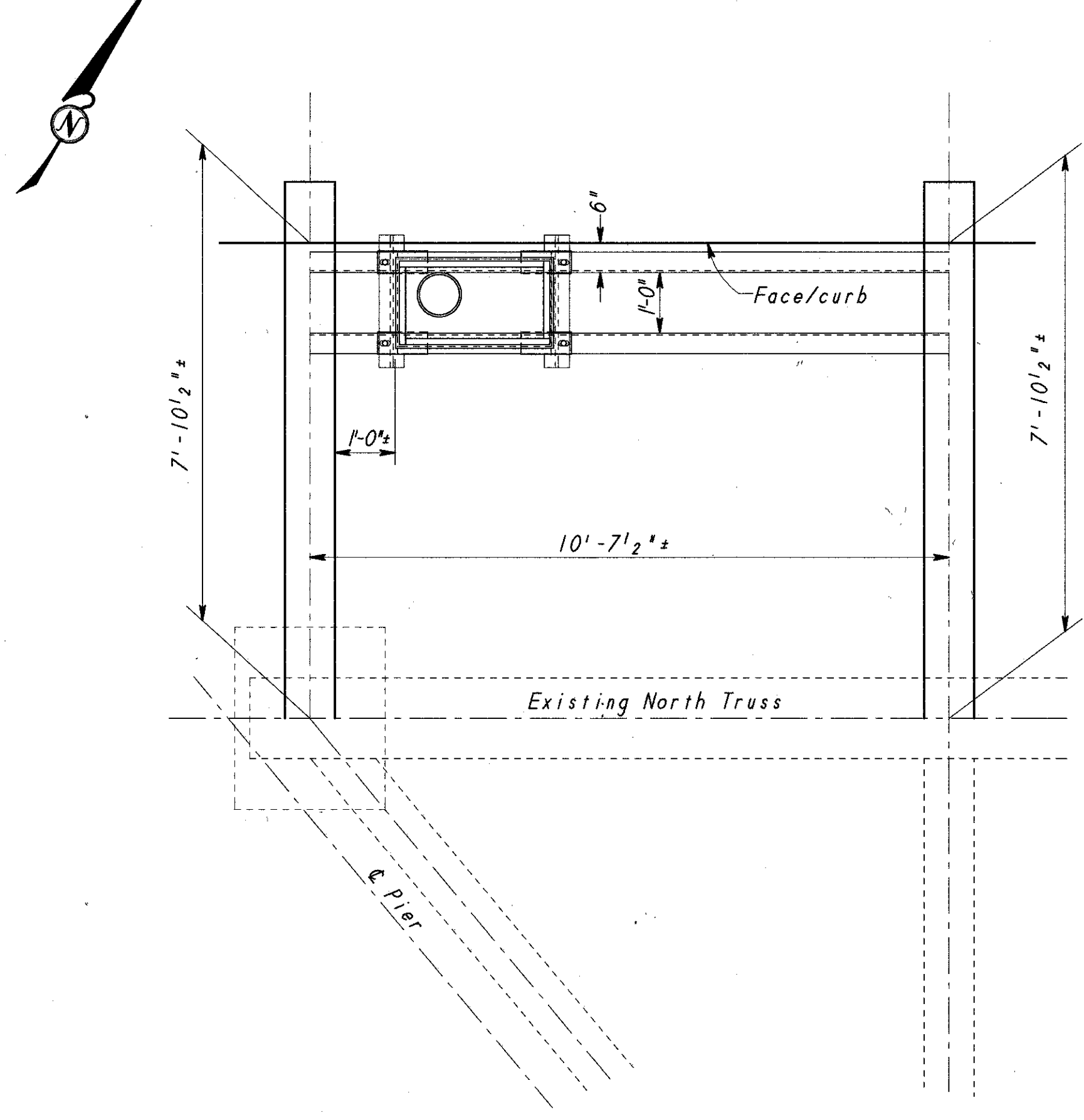
PIER No. 3 RIGHT

Pflum, Klausmeyer & Gehrmann		50/108
Consultants		OHIO
DRAINAGE DETAILS		
INLET LOCATIONS		
REAR ABUT. & PIER NO. 2 AND 3		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
		Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	JDR	WDD
CHECKED	REVIEWED	DATE
	IEH	April 96

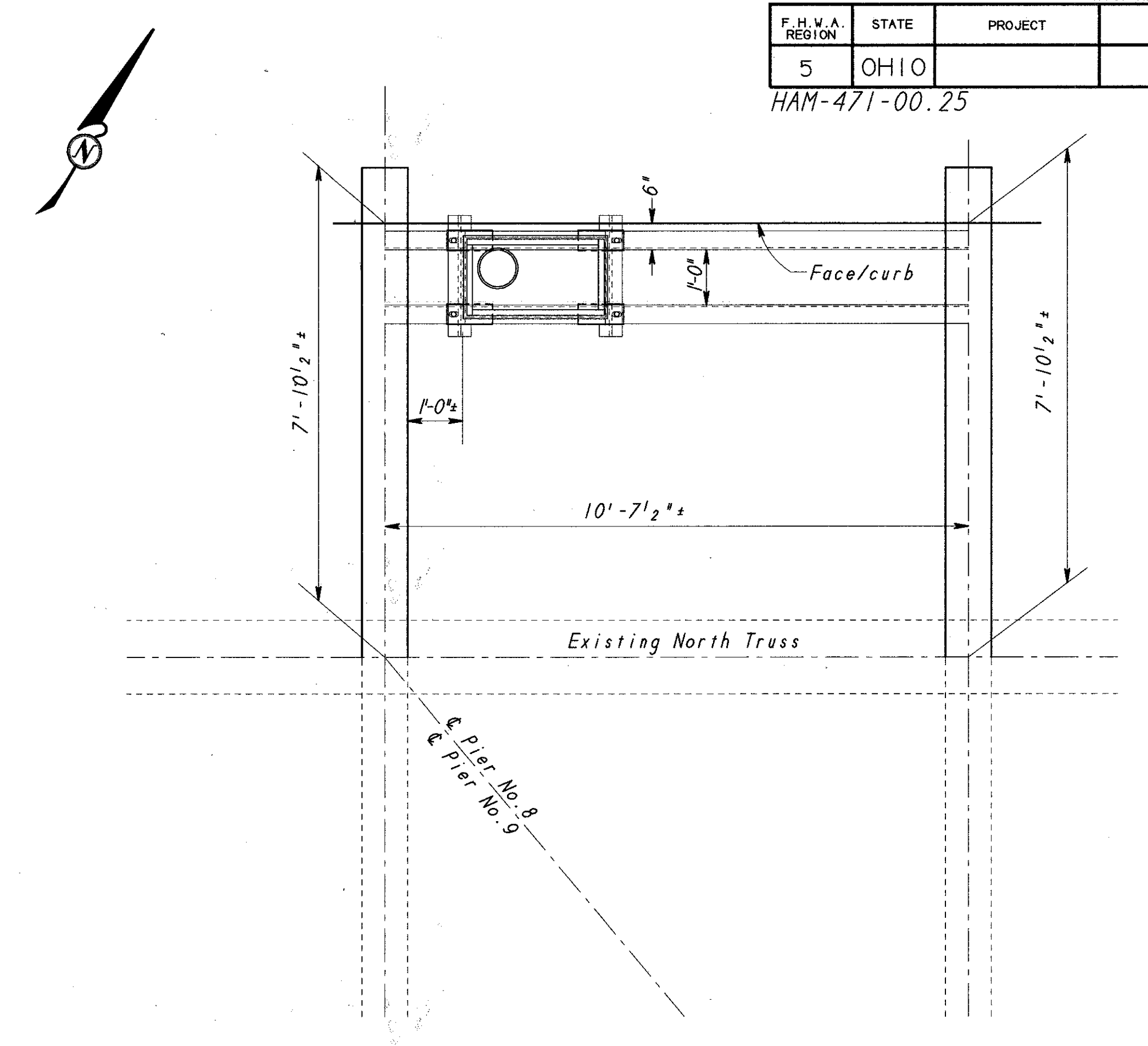
DRAWING Scale 2



PIER No. 4 LEFT



PIER No. 5 LEFT
PIER No. 7 LEFT
PIER No. 10 LEFT

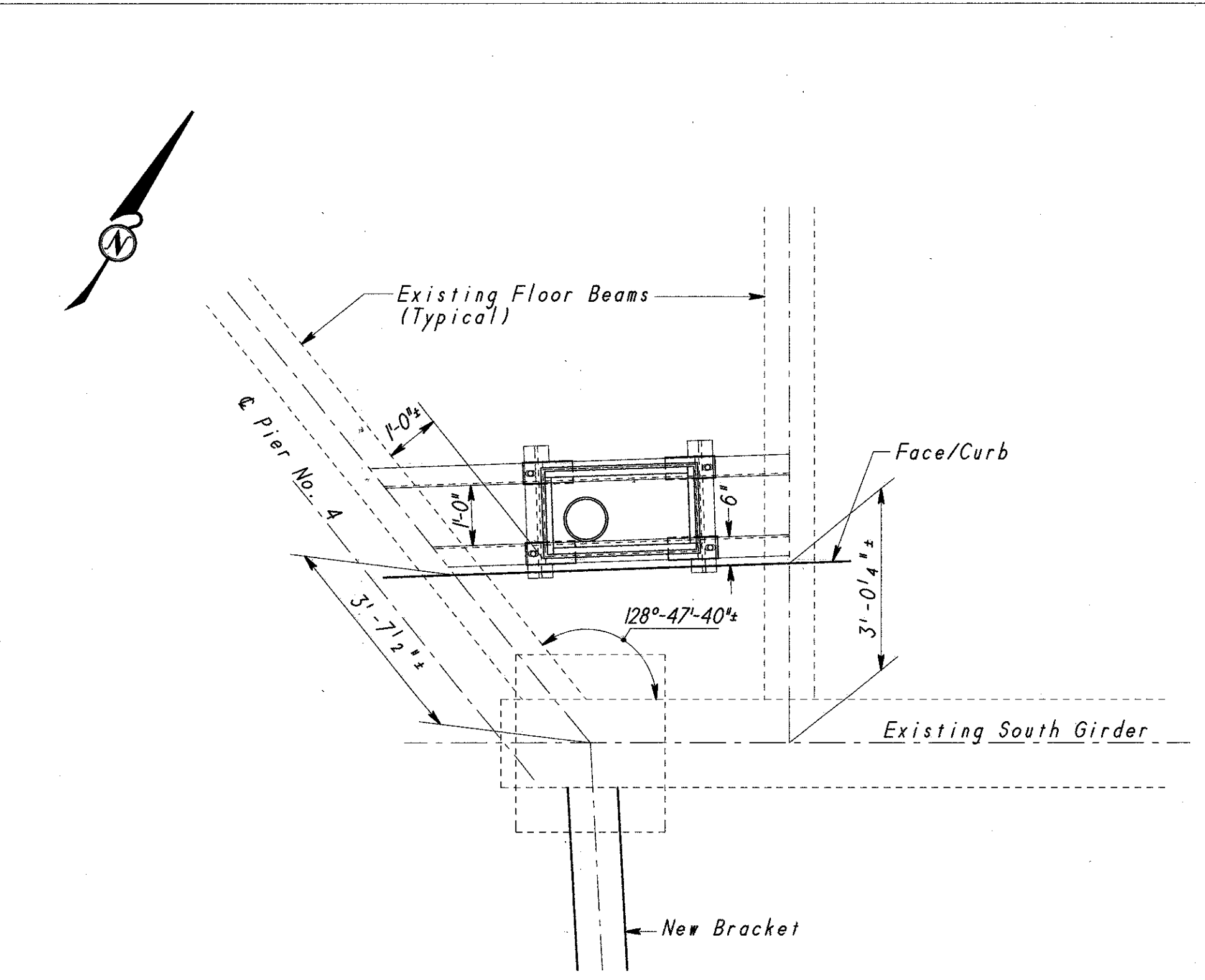


PIER No. 8 LEFT
PIER No. 9 LEFT

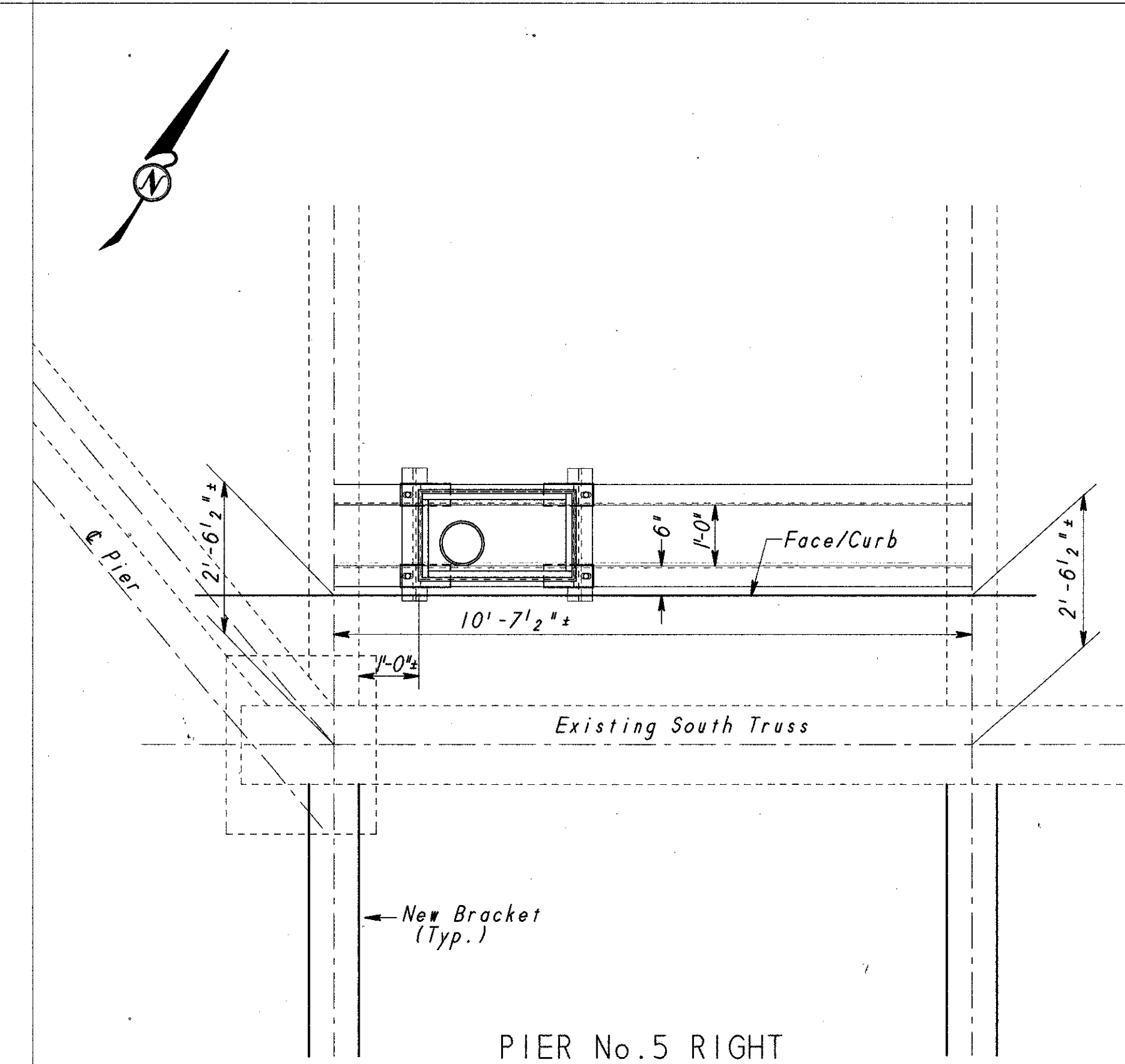
LEGEND

Existing Structure

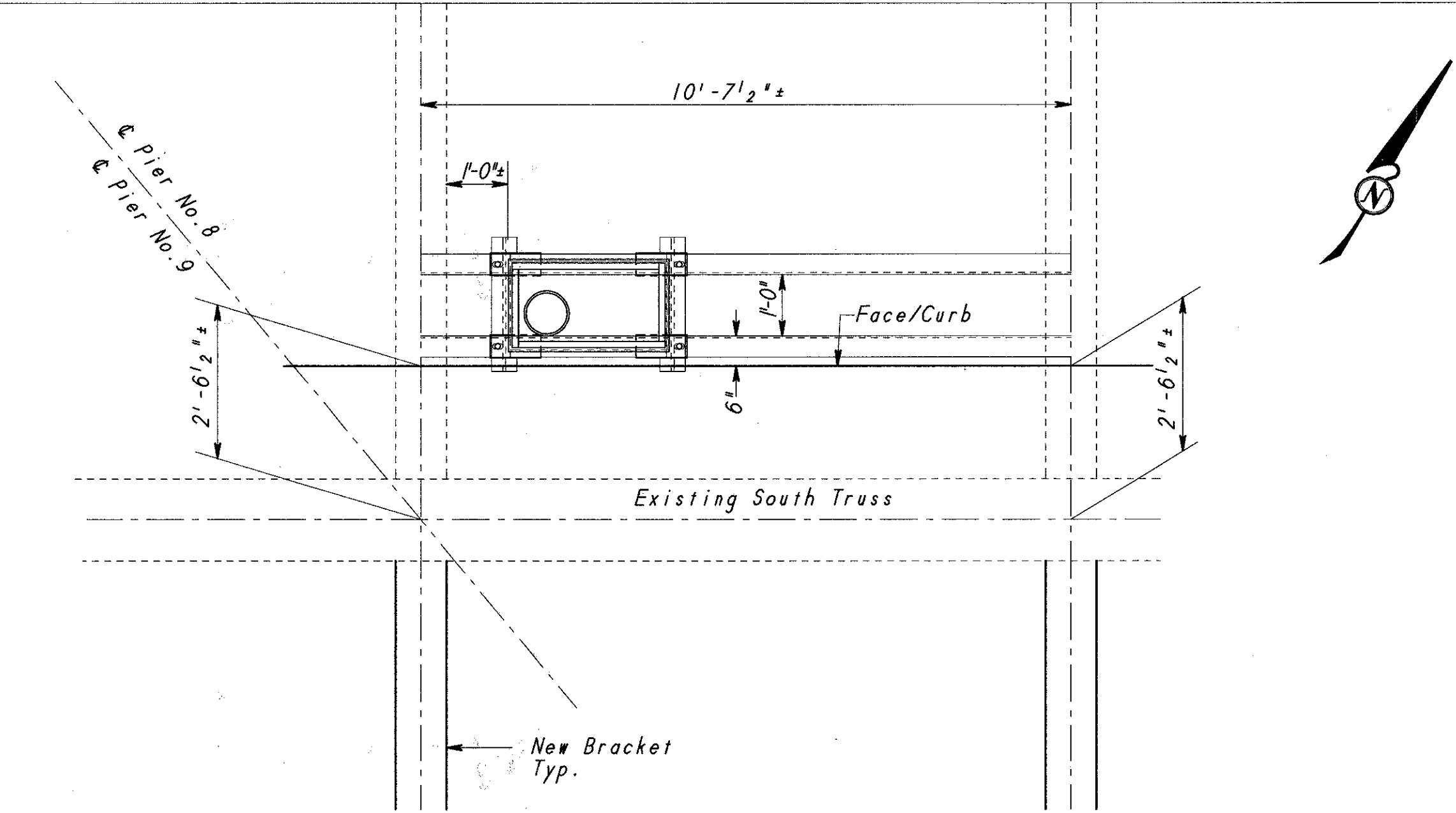
New Work



PIER No. 4 RIGHT



PIER No. 5 RIGHT
PIER No. 7 RIGHT
PIER No. 10 RIGHT

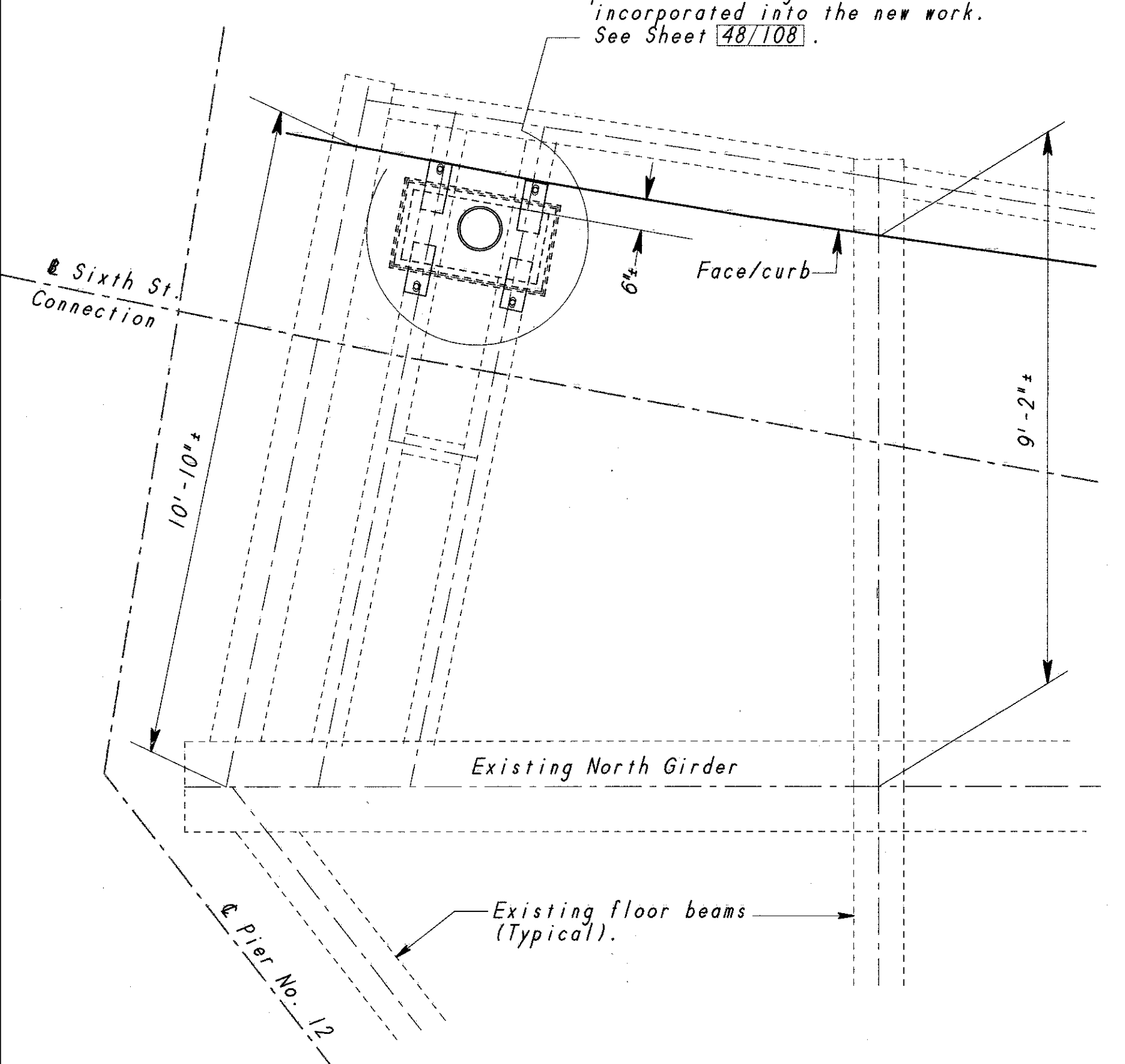


PIER No. 8 RIGHT
PIER No. 9 RIGHT

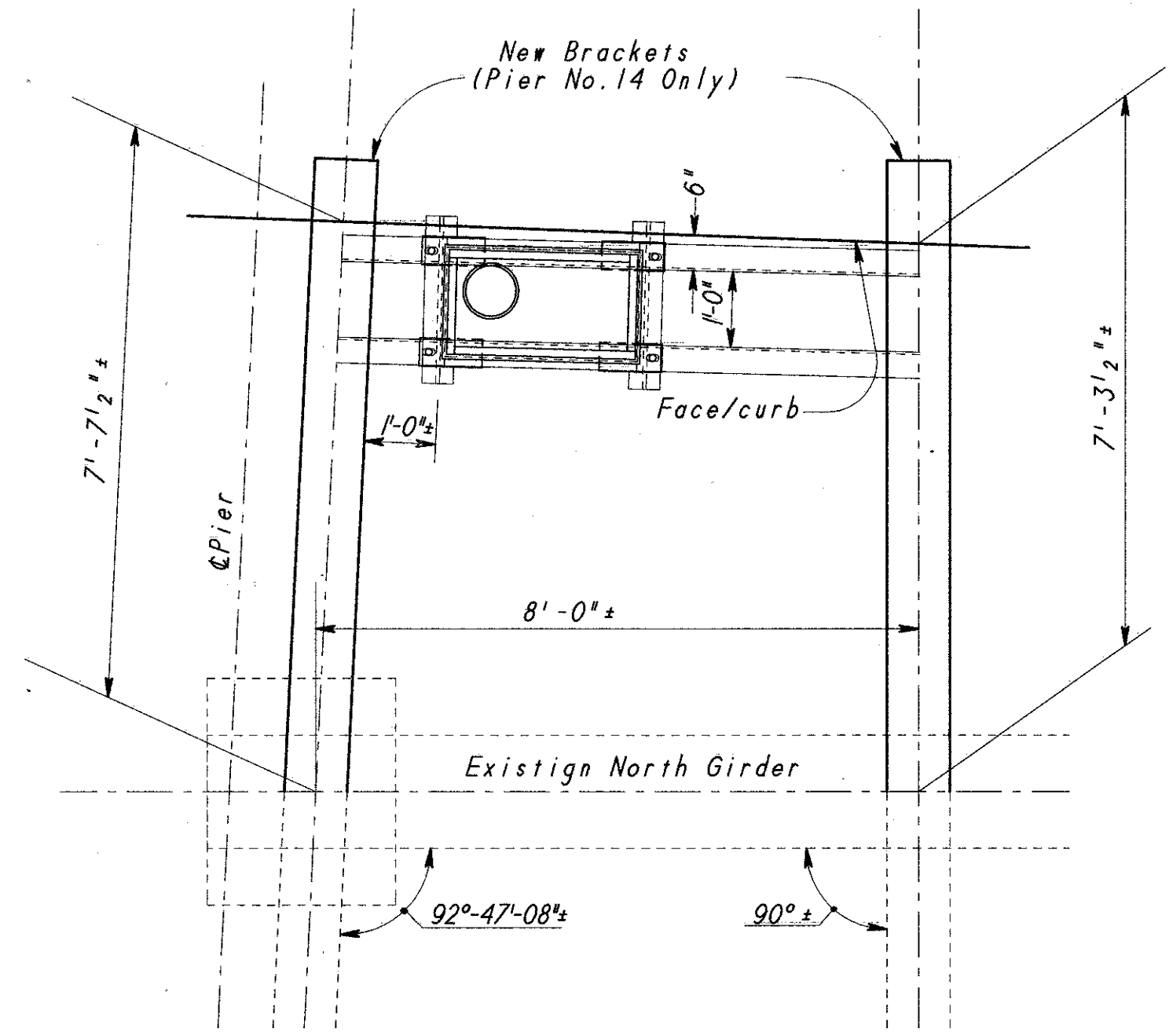
Plum, Klausmeier & Gehrum Consultants		51 / 108
OHIO		
DRAINAGE DETAILS		
INLET LOCATIONS		
(PIER No. 4 Thru PIER No. 10)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
		Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	JDR	WDD
CHECKED	REVIEWED	DATE
	IEH	April 96

DRA/INDT3 Scale 2.0

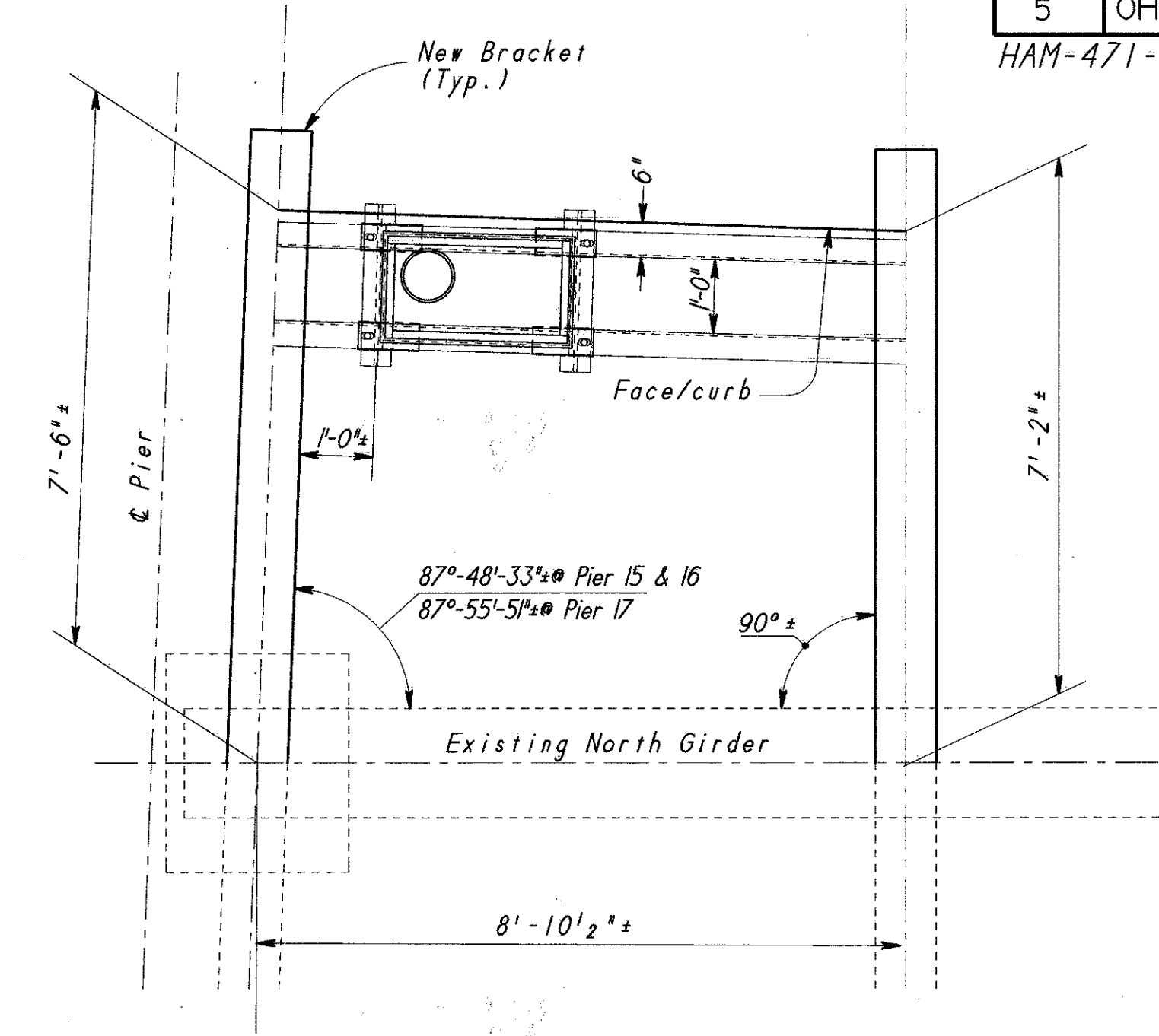
Existing drain inlet to be protected during removals, and incorporated into the new work. See Sheet 48/108.



PIER No. 12 LEFT



PIER No. 13 LEFT
PIER No. 14 LEFT

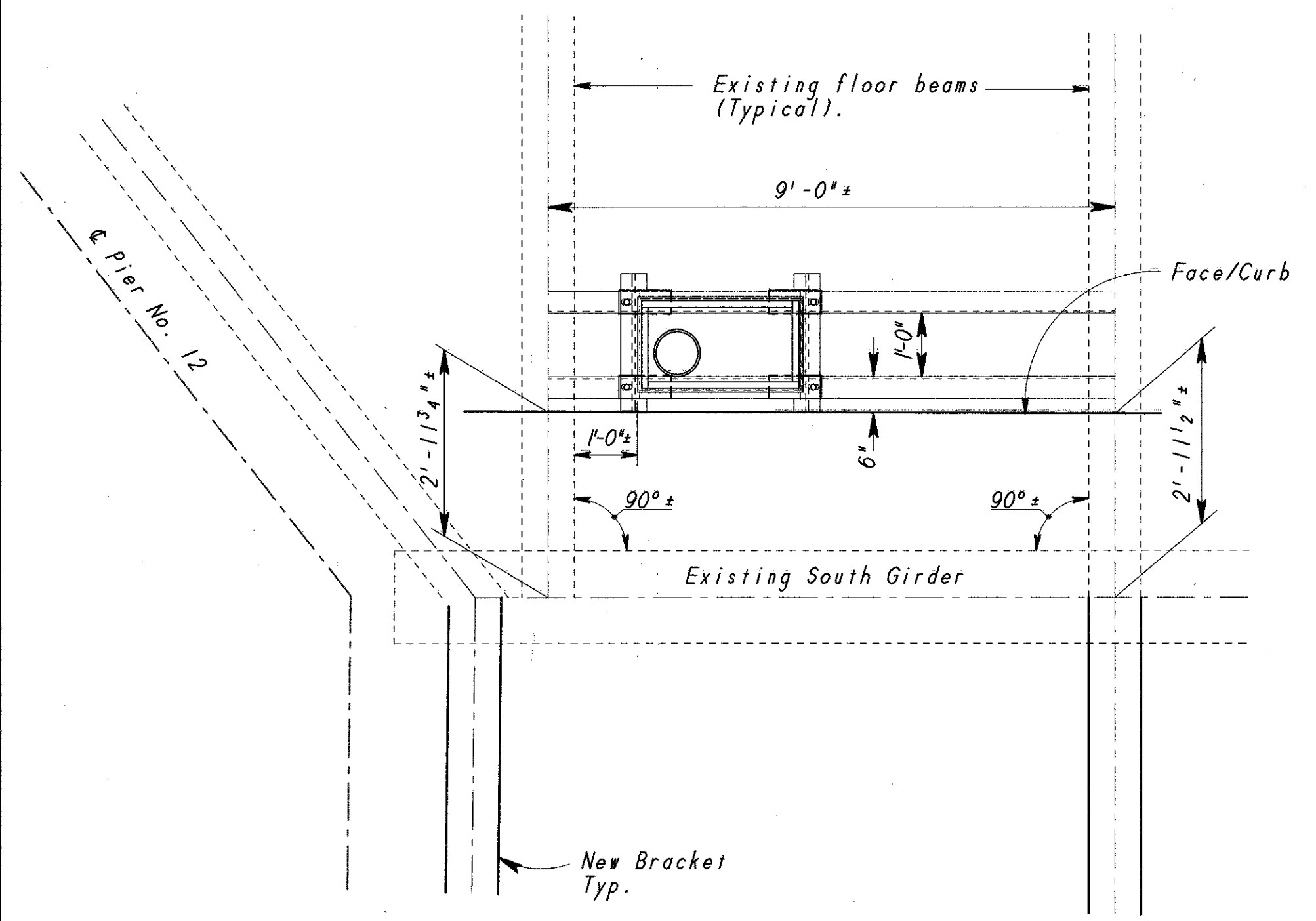


PIER No. 15 LEFT
PIER No. 16 LEFT
PIER No. 17 LEFT

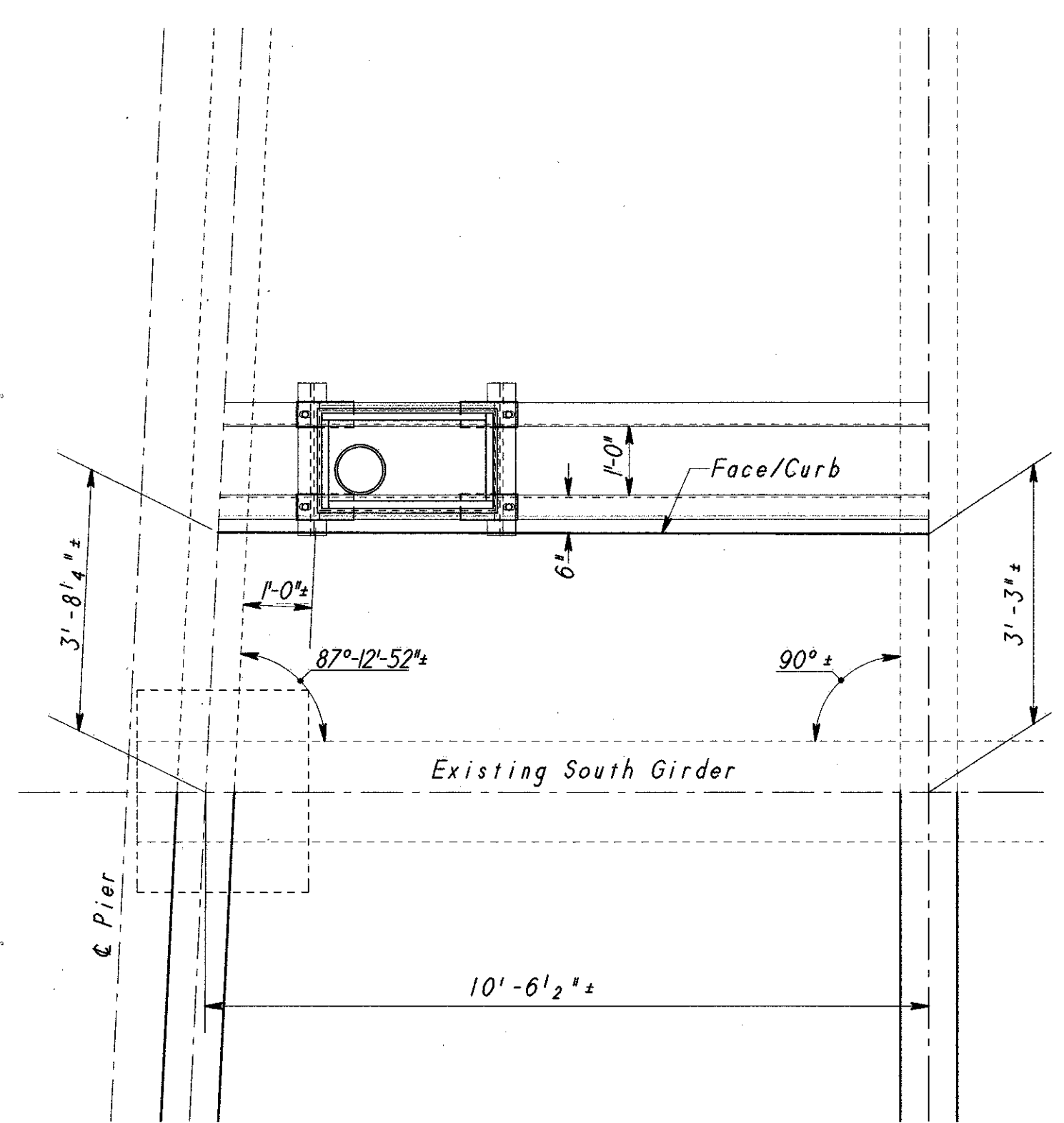
LEGEND

Existing Structure

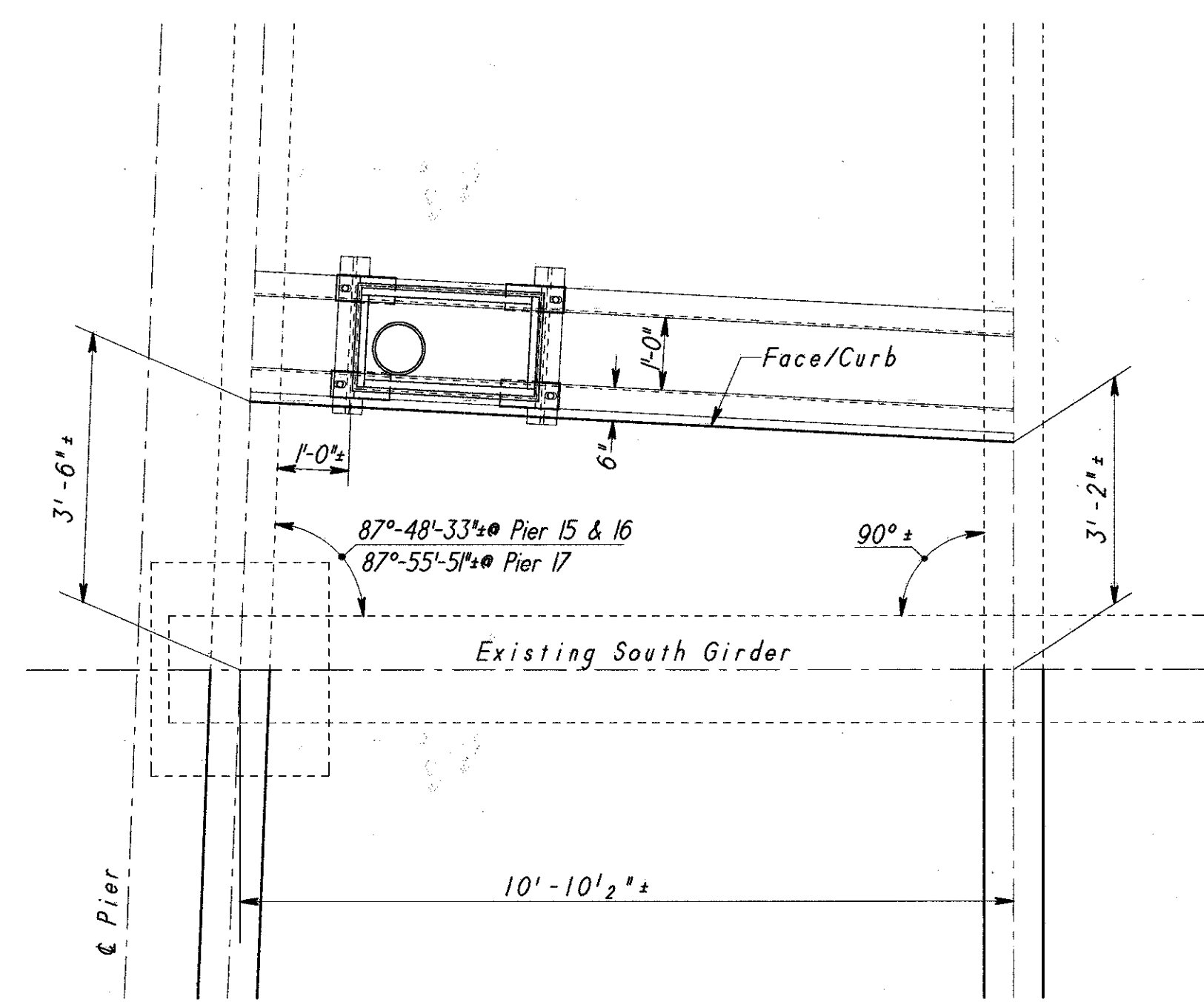
New Work



PIER No. 12 RIGHT



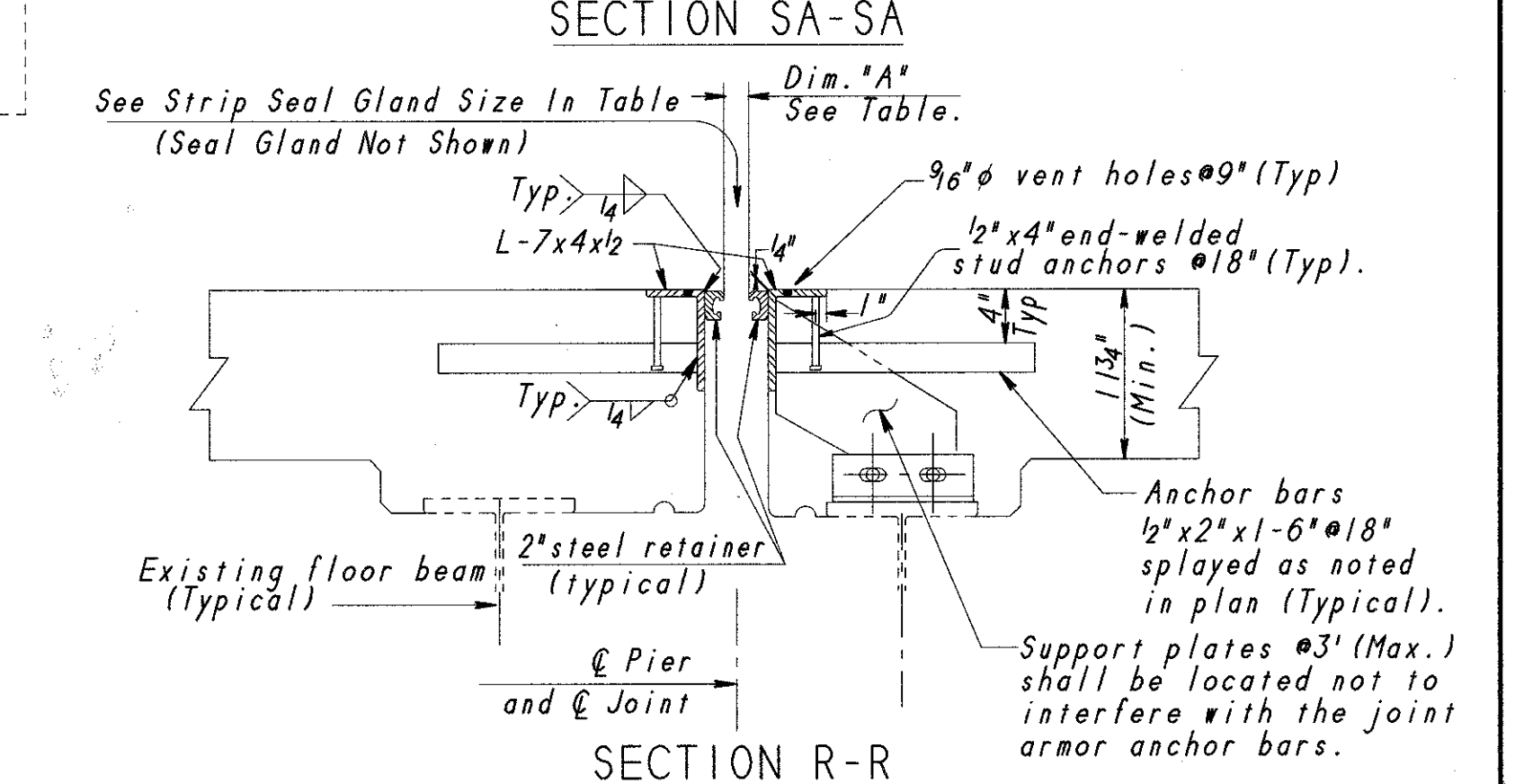
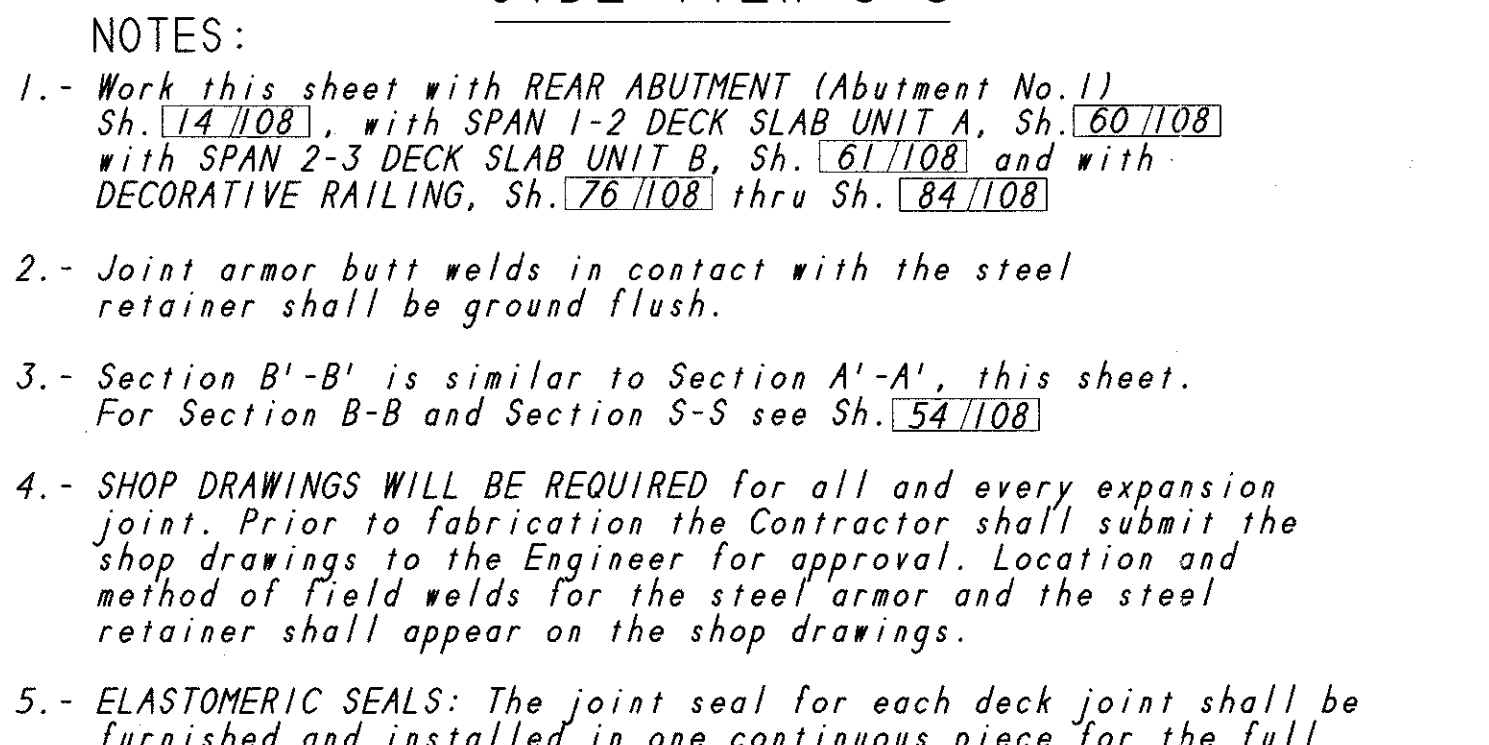
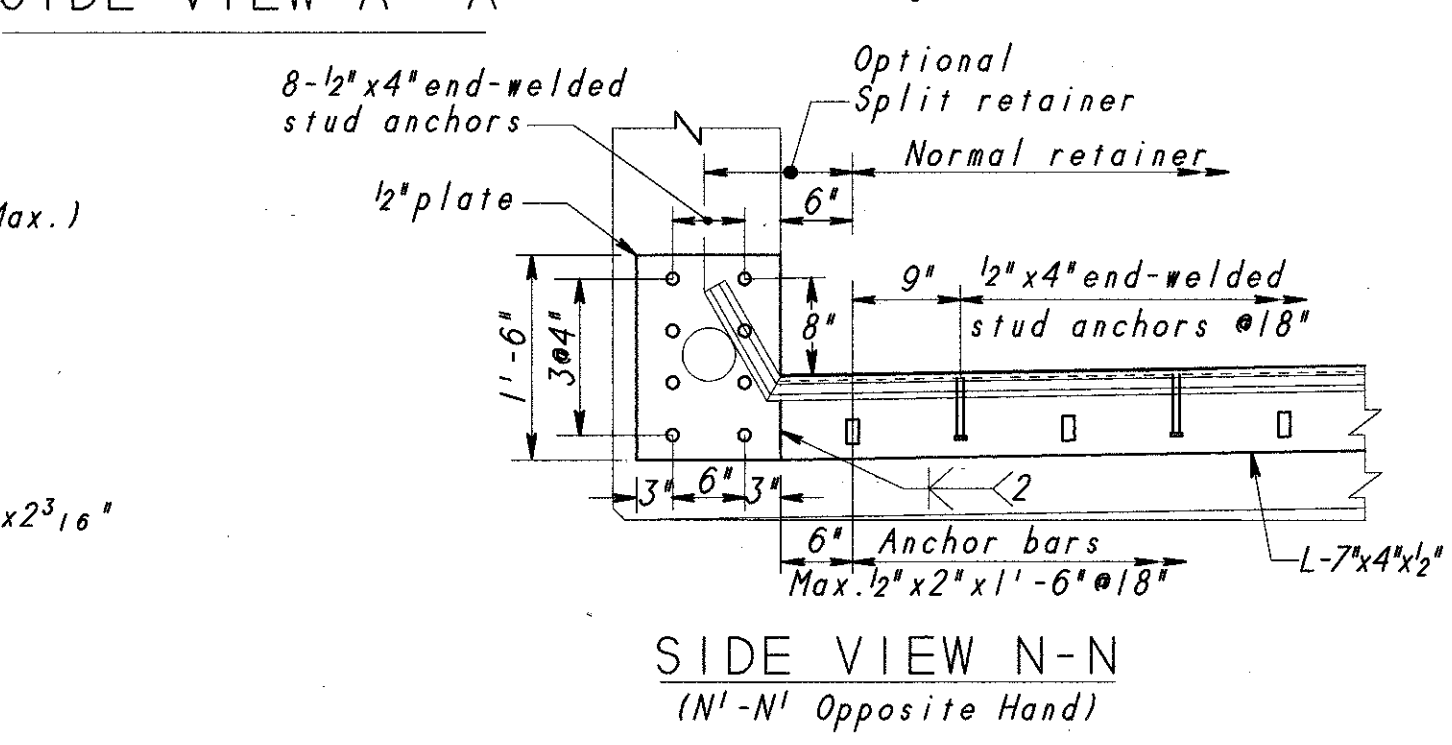
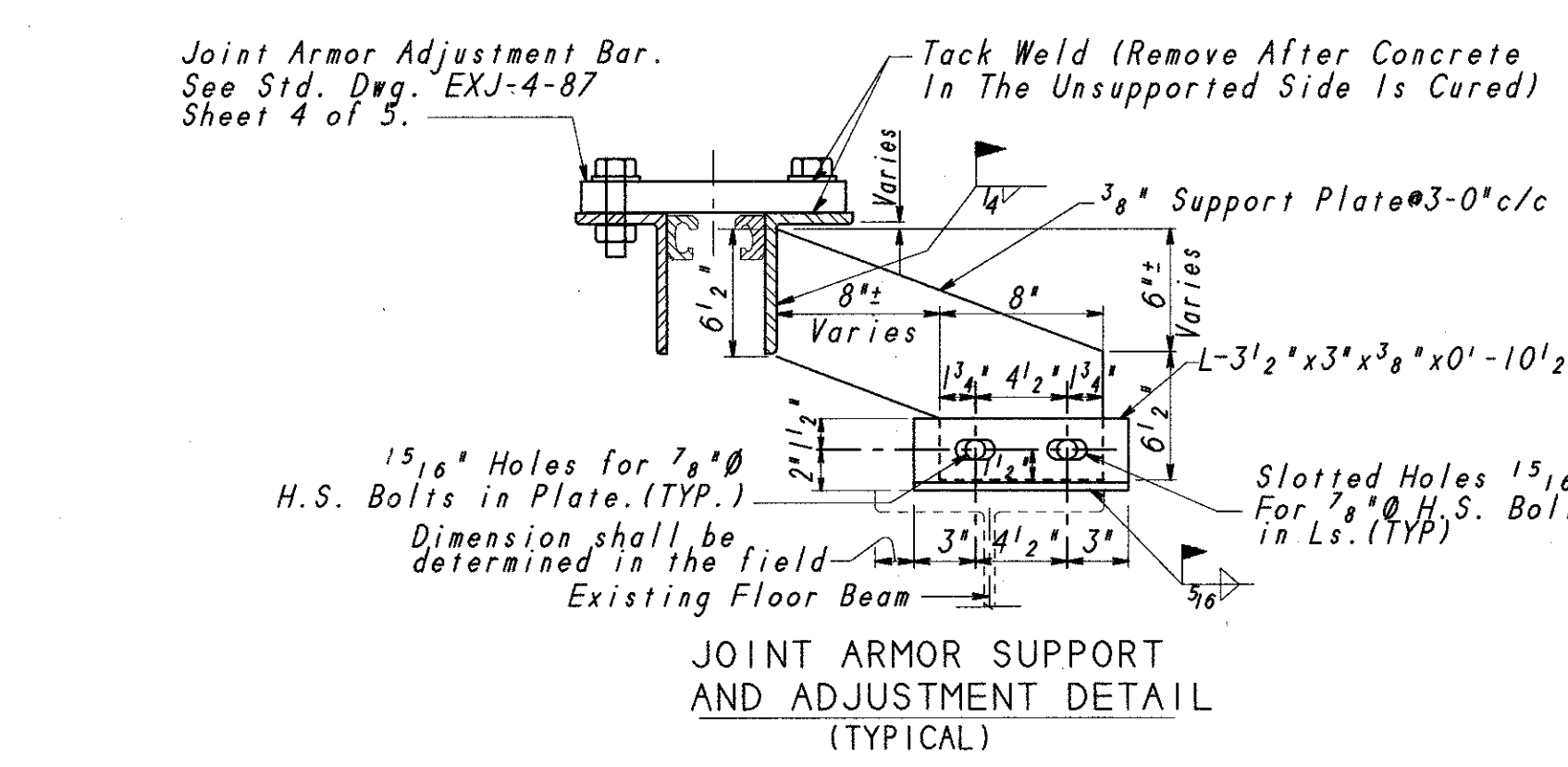
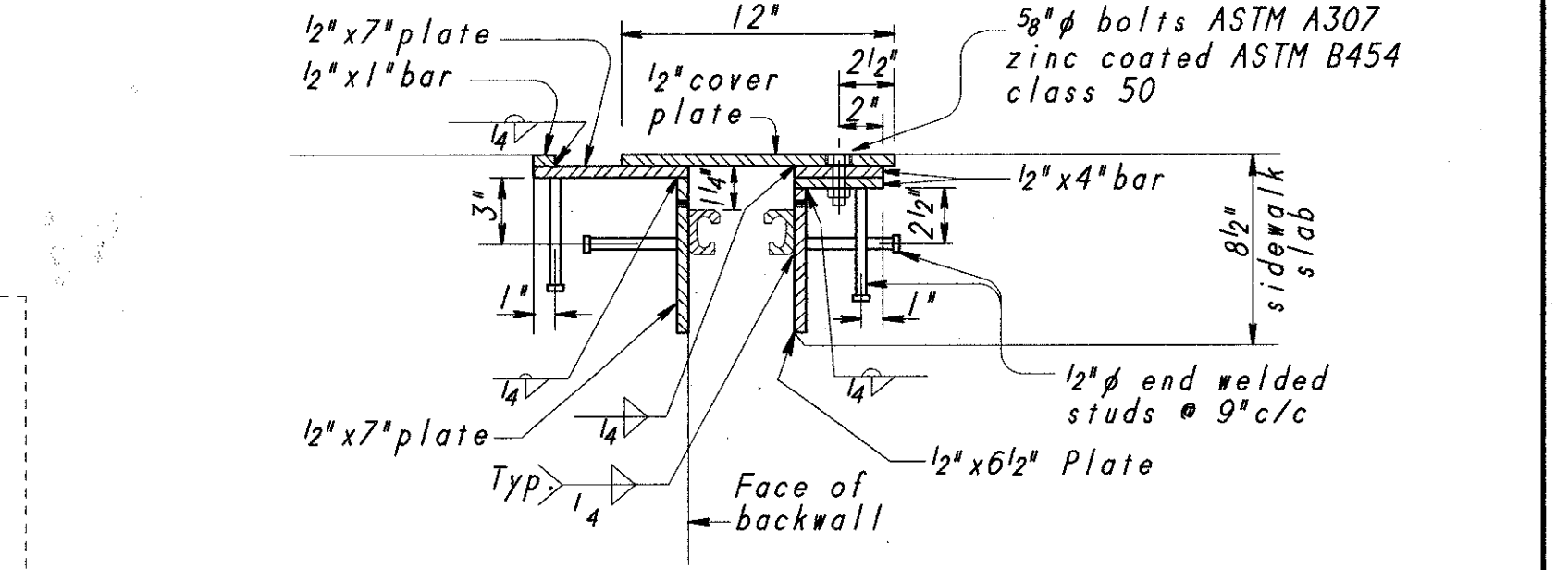
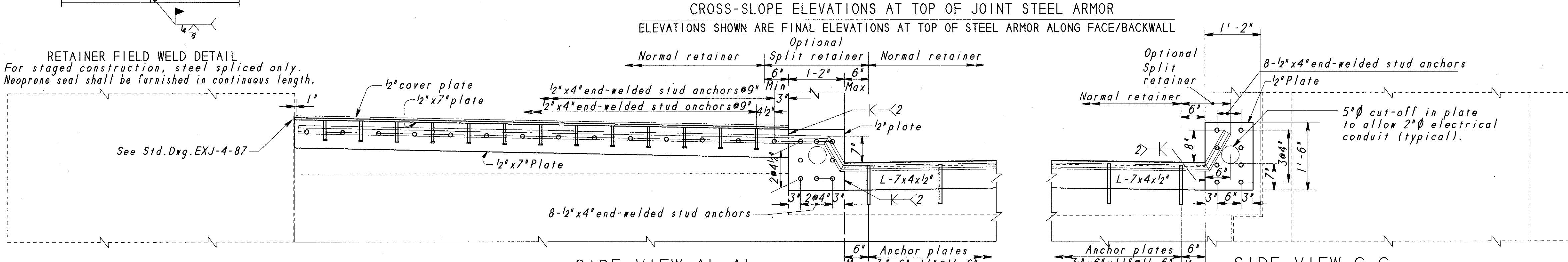
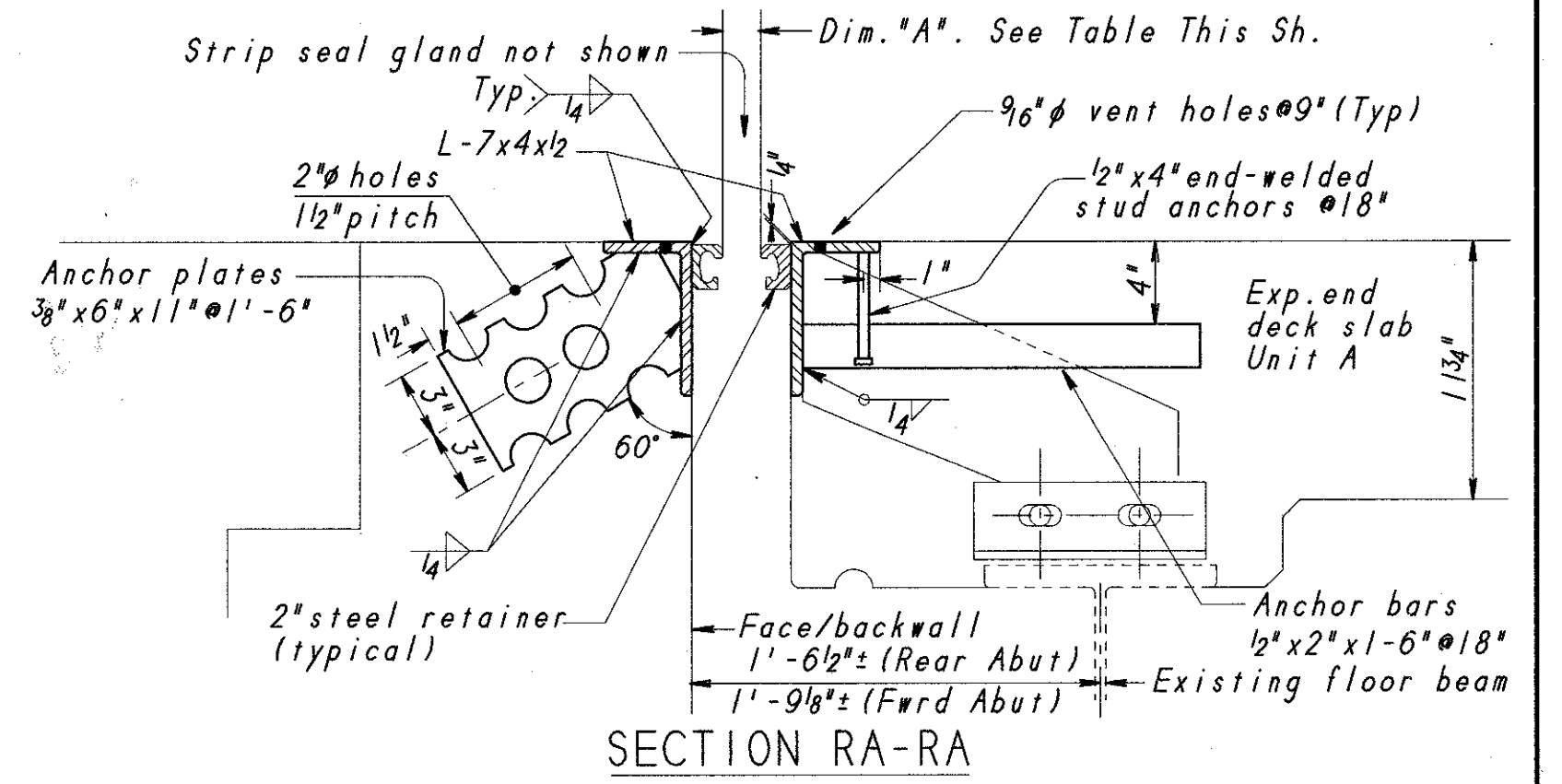
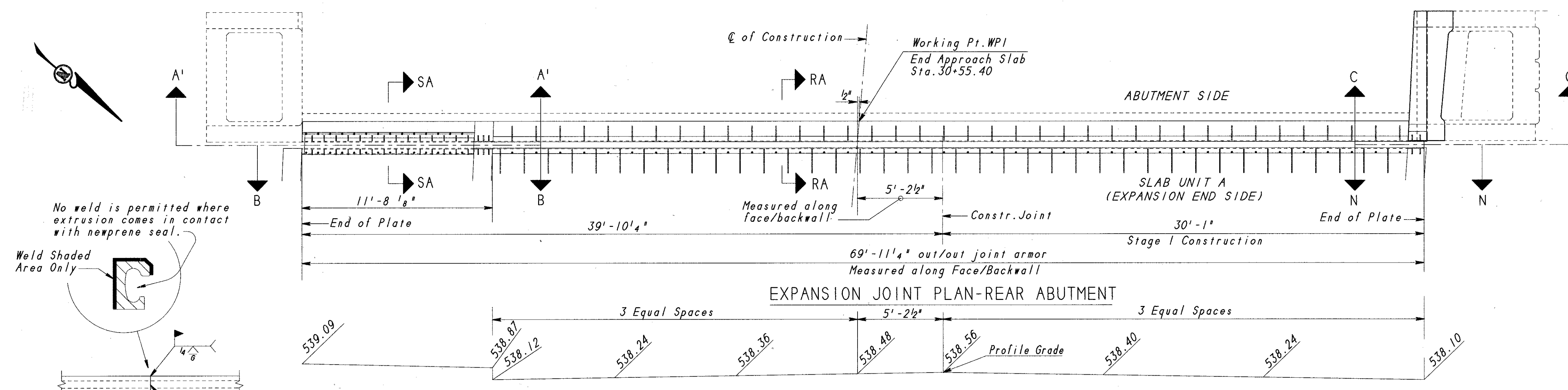
PIER No. 13 RIGHT
PIER No. 14 RIGHT



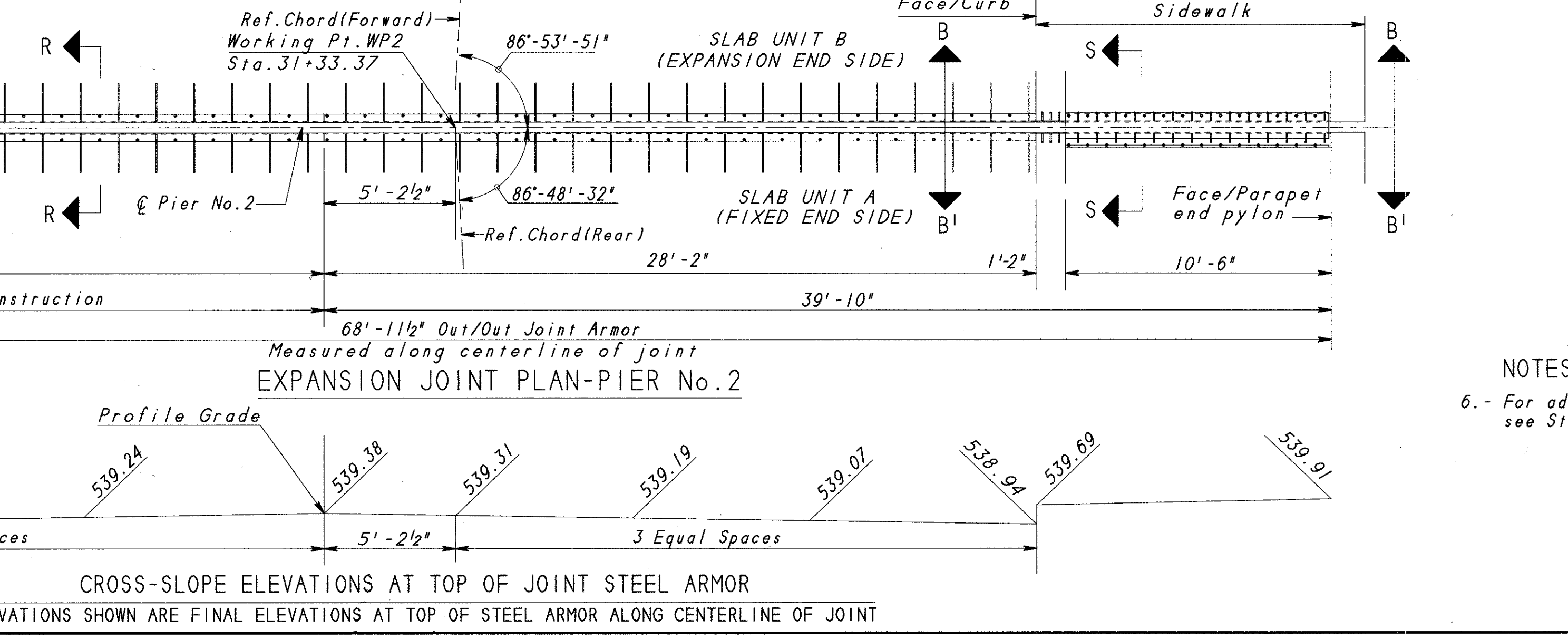
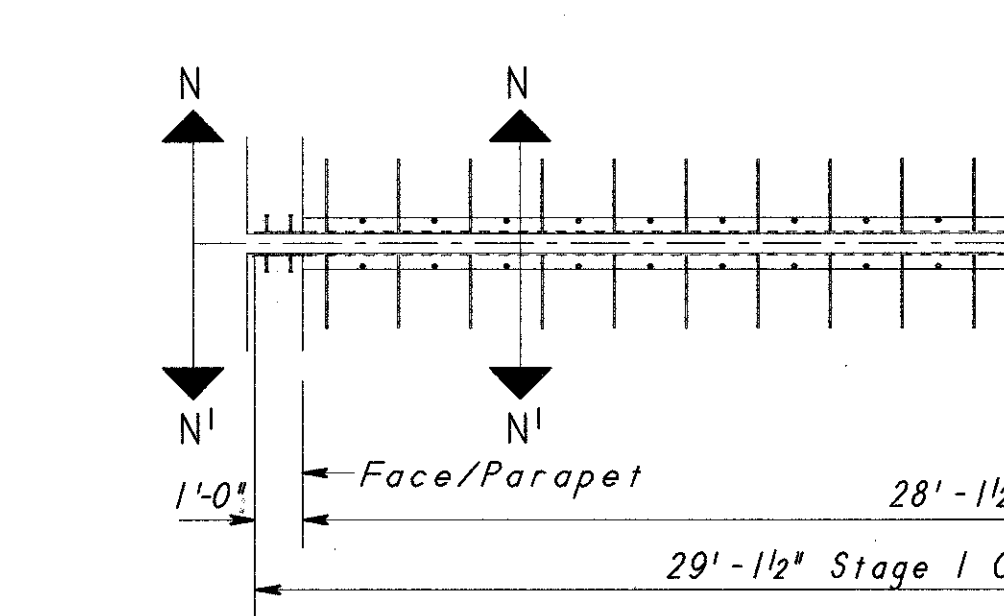
PIER No. 15 RIGHT
PIER No. 16 RIGHT
PIER No. 17 RIGHT

Pflum, Klausmeyer & Gehrum		52/108
Consultants		OHIO
DRAINAGE DETAILS		
INLET LOCATIONS		
(PIER No. 12 THRU PIER No. 17)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
		Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	JDR	WDD
TRACED	REVIEWED	DATE
	IEH	April 96

DRAINAGE Scale 2



- NOTES:
1. Work this sheet with REAR ABUTMENT (Abutment No. 1) Sh. [14/108], with SPAN 1-2 DECK SLAB UNIT A, Sh. [60/108] with SPAN 2-3 DECK SLAB UNIT B, Sh. [67/108] and with DECORATIVE RAILING, Sh. [76/108] thru Sh. [84/108]
 2. Joint armor butt welds in contact with the steel retainer shall be ground flush.
 3. Section B'-B' is similar to Section A'-A', this sheet. For Section B-B and Section S-S see Sh. [54/108]
 4. SHOP DRAWINGS WILL BE REQUIRED for all and every expansion joint. Prior to fabrication the Contractor shall submit the shop drawings to the Engineer for approval. Location and method of field welds for the steel armor and the steel retainer shall appear on the shop drawings.
 5. ELASTOMERIC SEALS: The joint seal for each deck joint shall be furnished and installed in one continuous piece for the full bridge width after both stages of deck joint armor installation is complete.



LOCATION	STRIP SEAL GLAND MOVEMENT RATING.	DIMENSION "A"						
		30°F	40°F	50°F	60°F	70°F	80°F	90°F
Rear Abut.	3"	1'5 1/8"	1'7 3/8"	1'13 1/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"
Pier No. 2	3"	1'5 1/8"	1'7 3/8"	1'13 1/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"
Pier No. 3	3"	1'5 1/8"	1'7 3/8"	1'13 1/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"
Pier No. 4	3"	1'5 1/8"	1'7 3/8"	1'13 1/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"
Pier No. 5	4"	2'3 3/8"	2'1 3/8"	2'1 3/8"	2"	1'7 3/8"	1'3 3/8"	1'5 3/8"
Pier No. 7	5"	2'13 1/8"	2'5 3/8"	2'16 1/8"	2'1 3/8"	2'16 1/8"	2'7 3/8"	2'11 1/8"
Pier No. 10	3"	1'7 3/8"	1'13 1/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"	1'7 3/8"	1'3 3/8"
Pier No. 12	4"	2'3 3/8"	2'1 3/8"	2"	1'7 3/8"	1'3 3/8"	1'5 3/8"	1'3 3/8"
Pier No. 13	3"	1'5 1/8"	1'7 3/8"	1'3 3/8"	1'5 3/8"	1'11 1/8"	1'3 3/8"	1'5 1/8"
Pier No. 14	3"	1'13 1/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"	1'2 3/8"	1'7 1/8"
Pier No. 15	3"	1'3 3/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"	1'2 3/8"	1'2 3/8"
Pier No. 16	3"	1'3 3/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"	1'2 3/8"	1'2 3/8"
Pier No. 17	3"	1'3 3/8"	1'3 3/8"	1'11 1/8"	1'5 3/8"	1'9 1/8"	1'2 3/8"	1'2 3/8"
Fwd Abut.	3"	1'2" @ Any Temp.						

- NOTES:
6. For additional details and notes see Std. Dwg. EXJ-4-87.

Plum, Klausmeier & Gehrum
Consultants

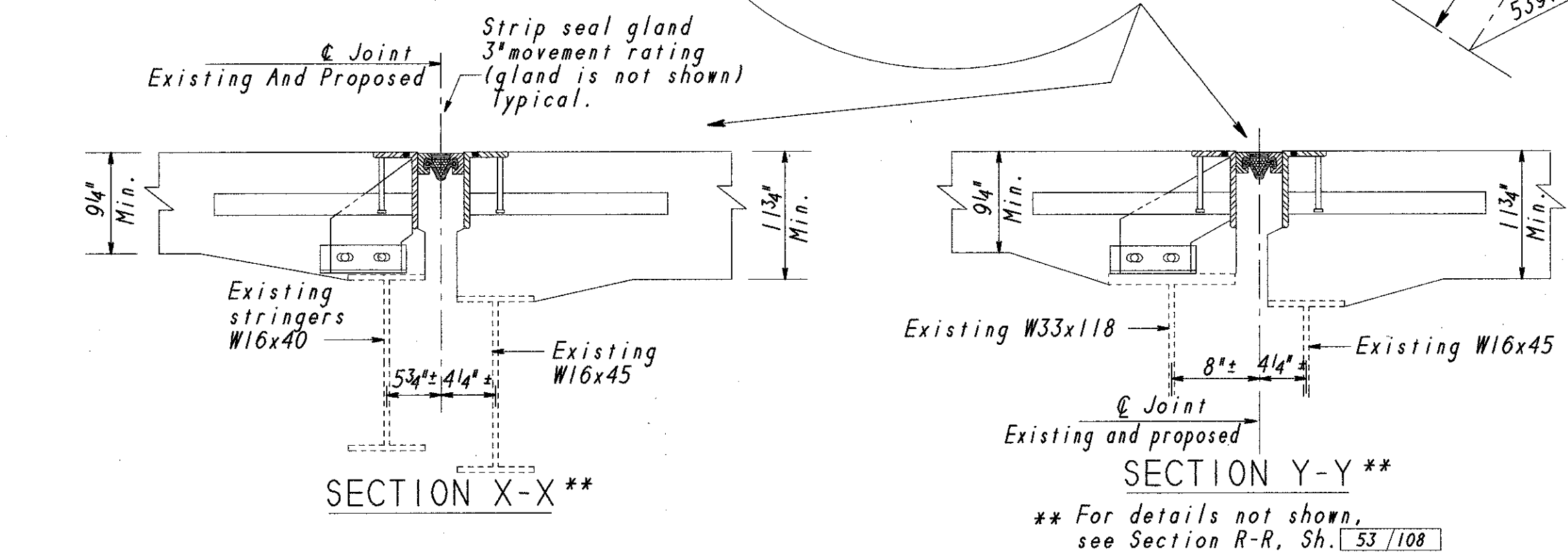
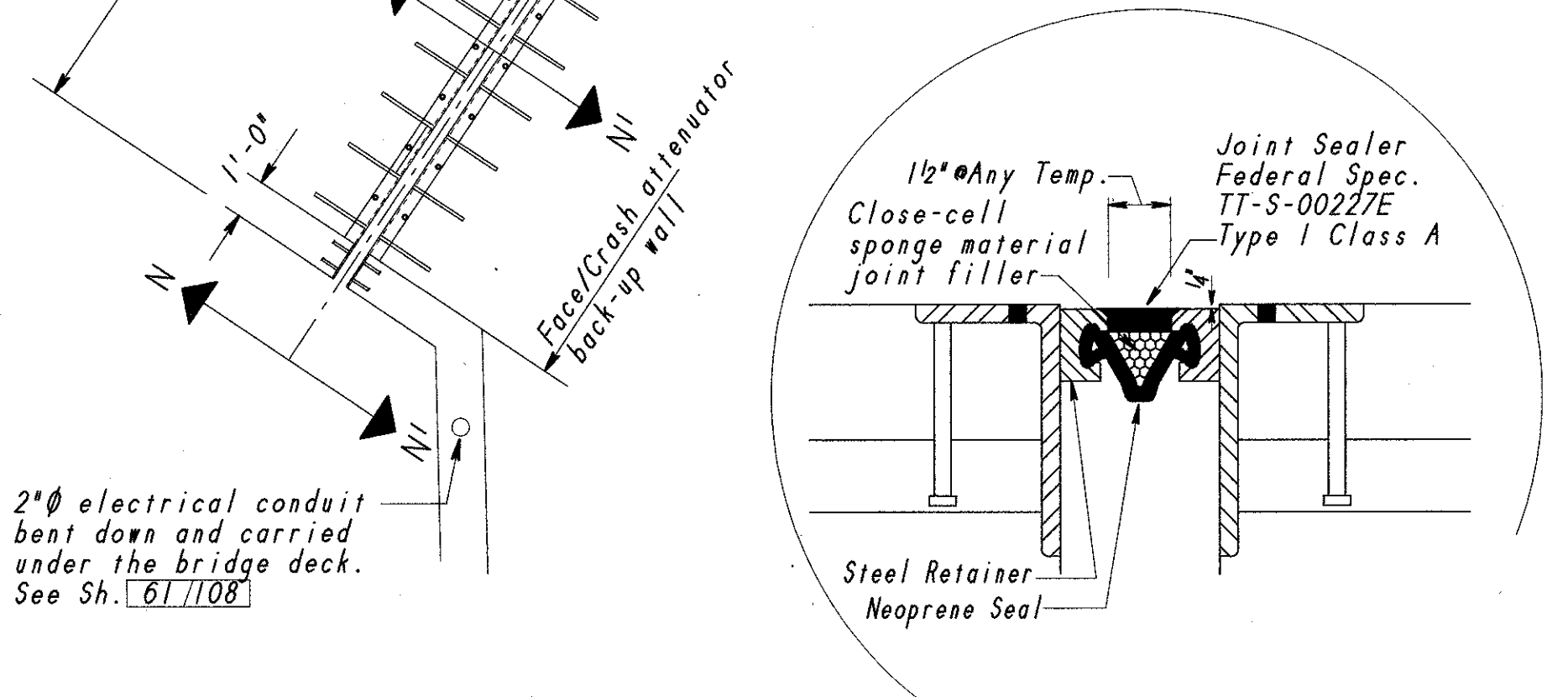
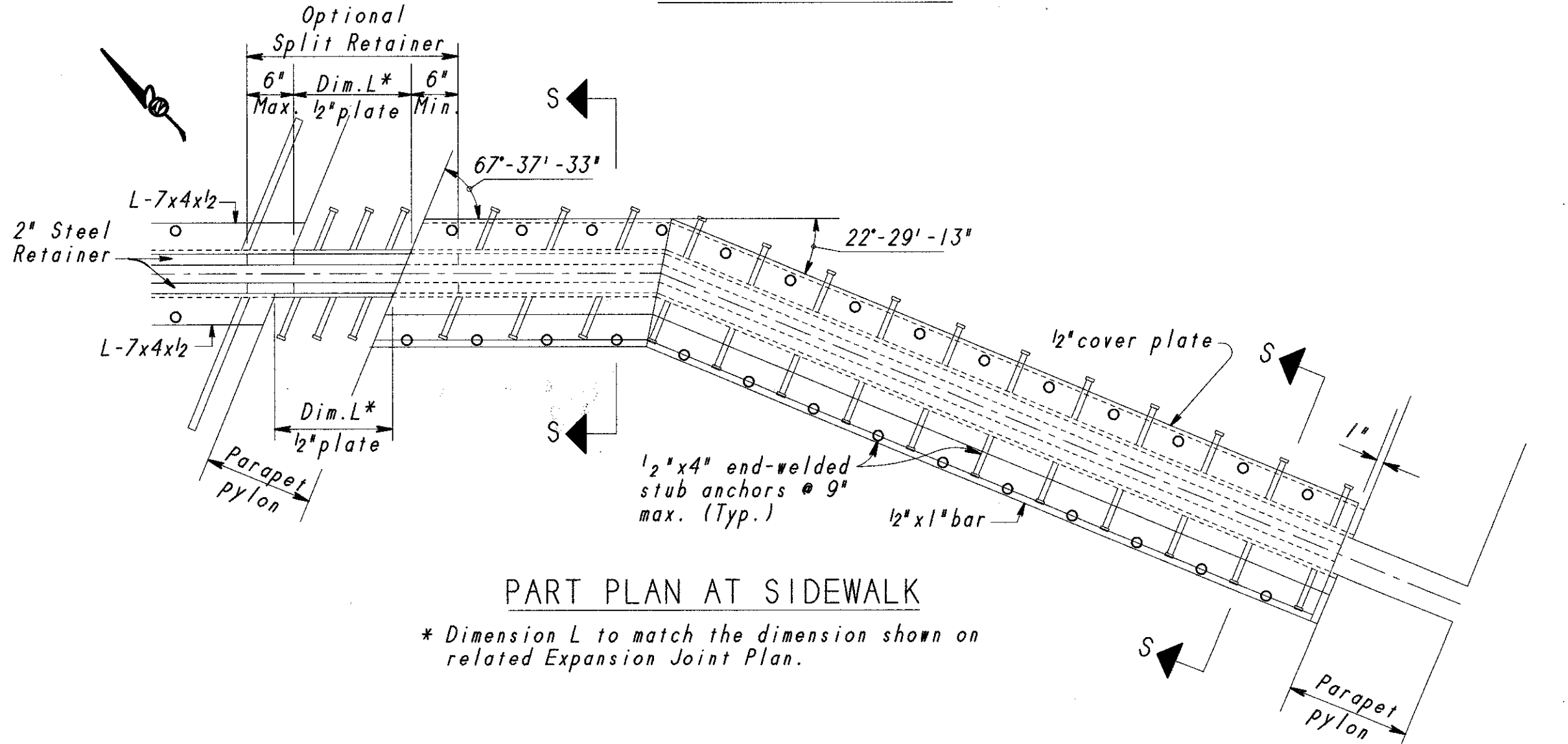
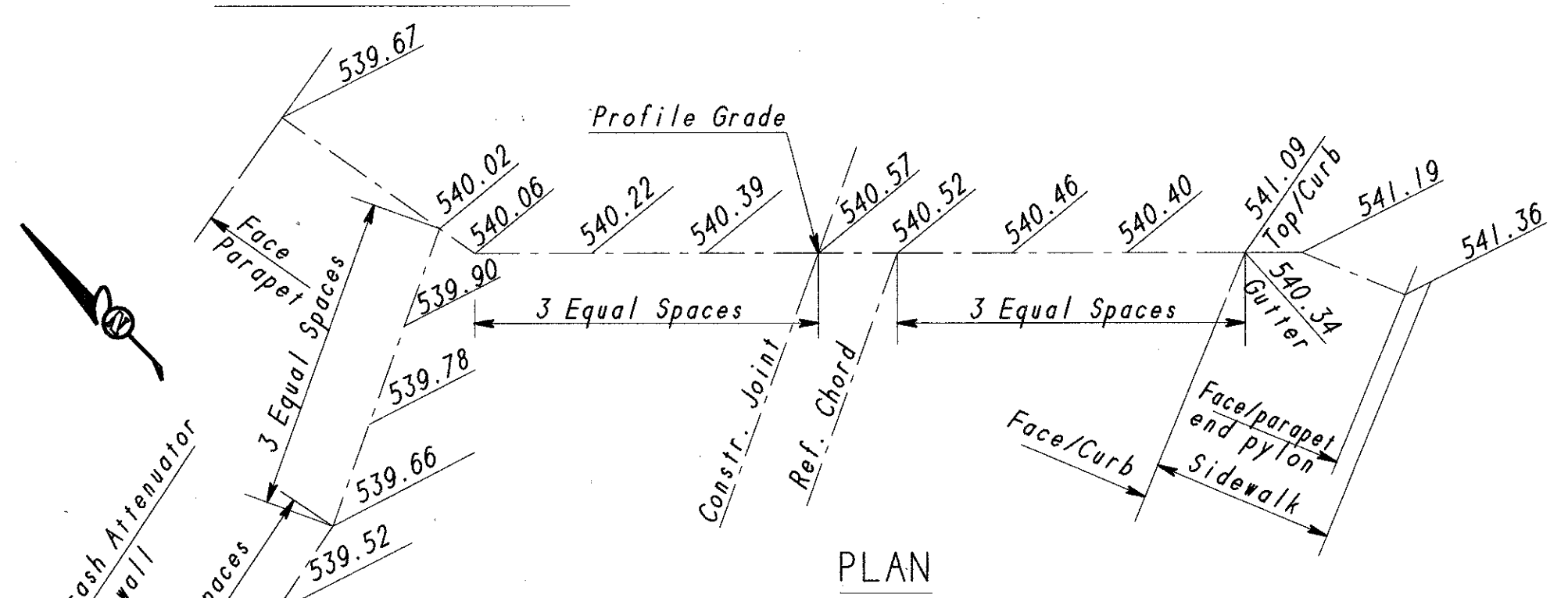
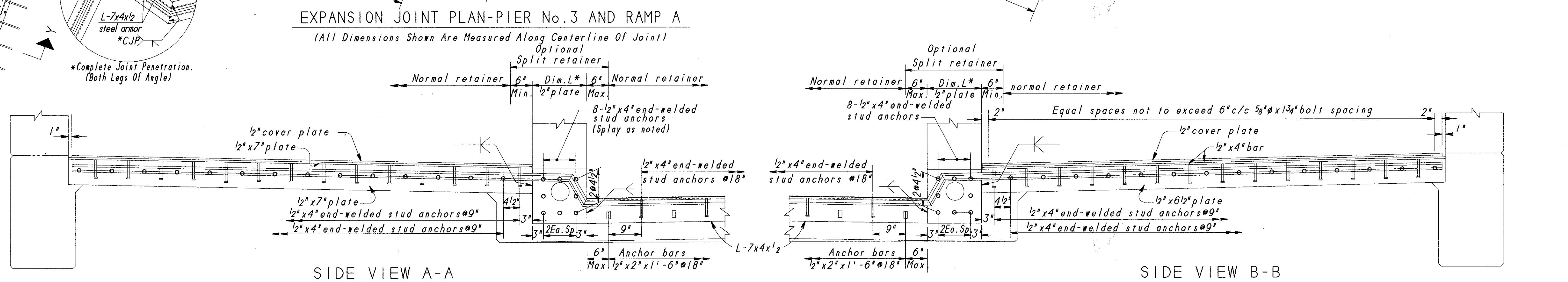
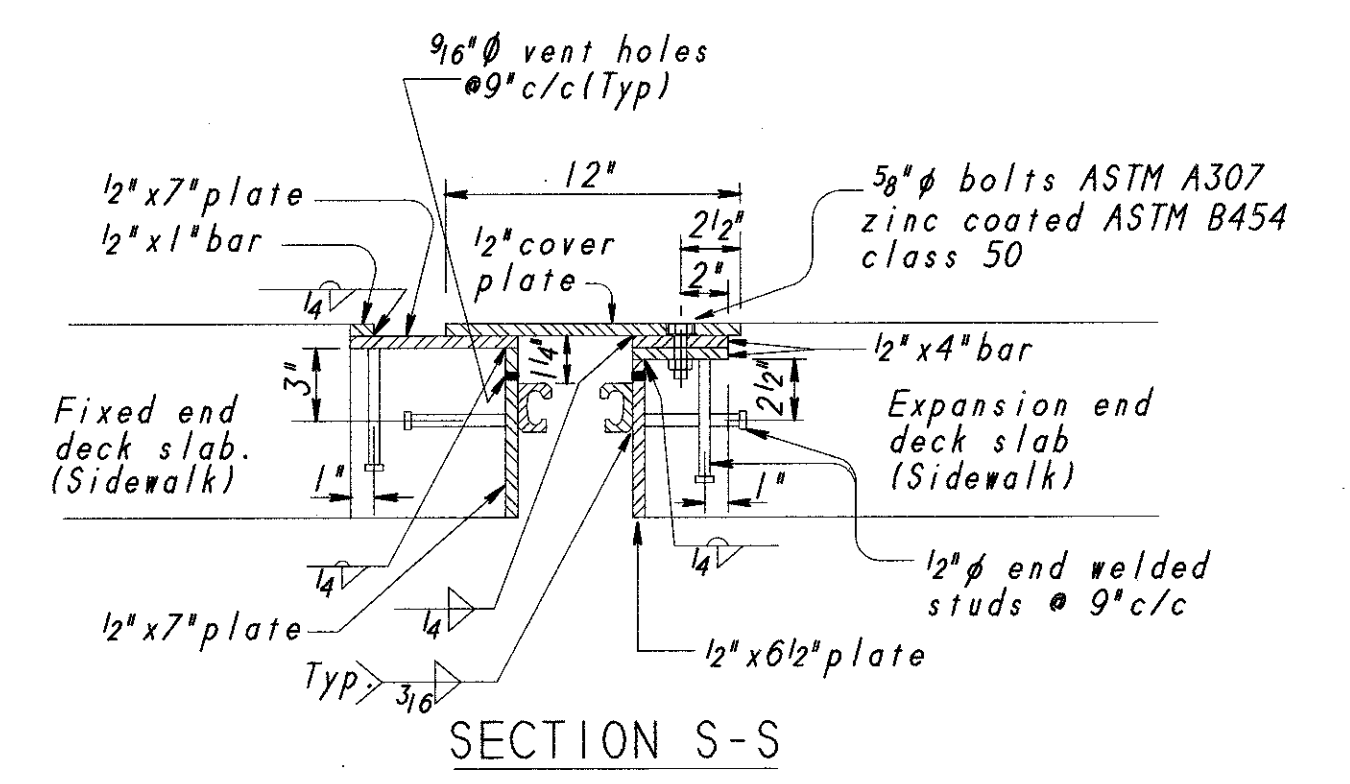
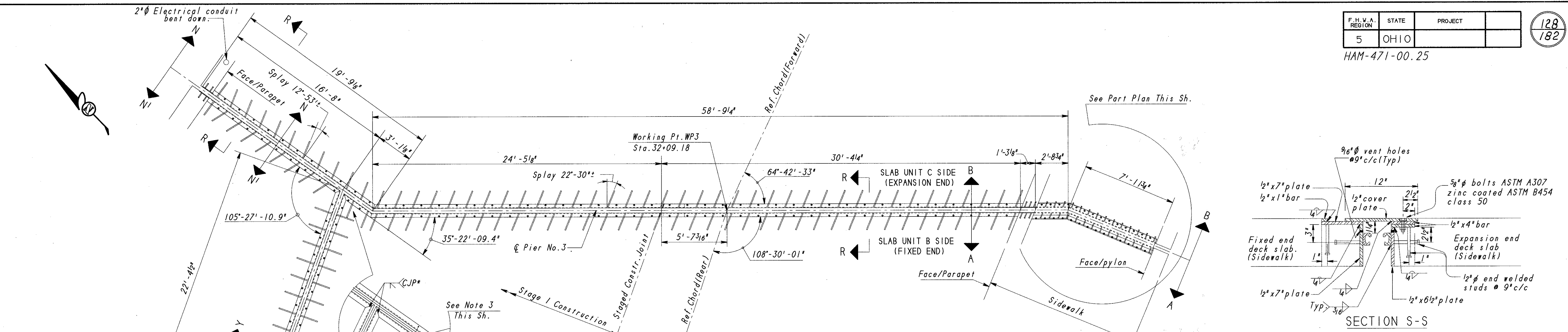
53/108

SUPERSTRUCTURE DETAILS
STRIP SEAL EXPANSION JOINT
AT REAR ABUTMENT (No. 1)
AND AT PIER No. 2
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)
Sta. 30+55.40
Sta. 47+16.19

HAMILTON COUNTY

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED/PLF		WDD	IEH	April 96	

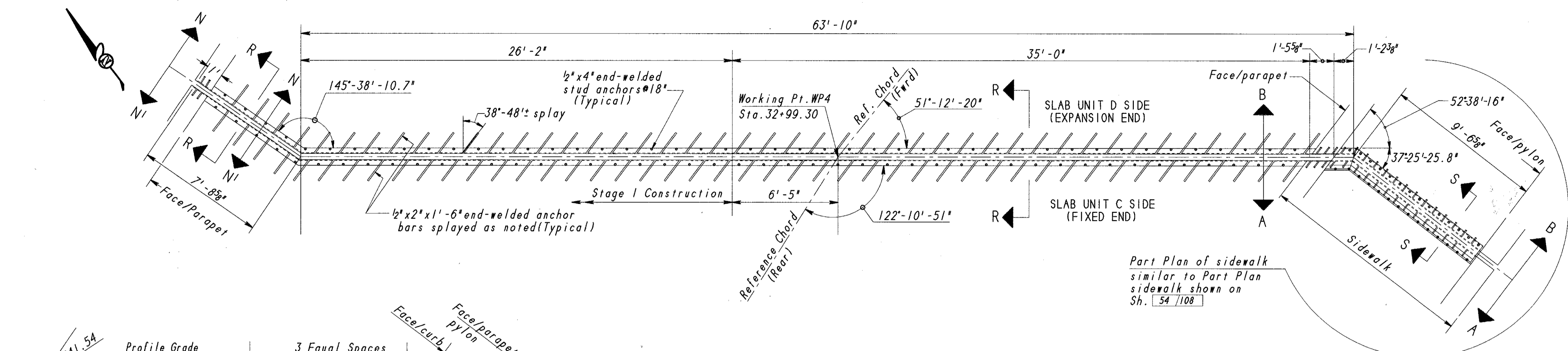
JOINTRA Scale 4



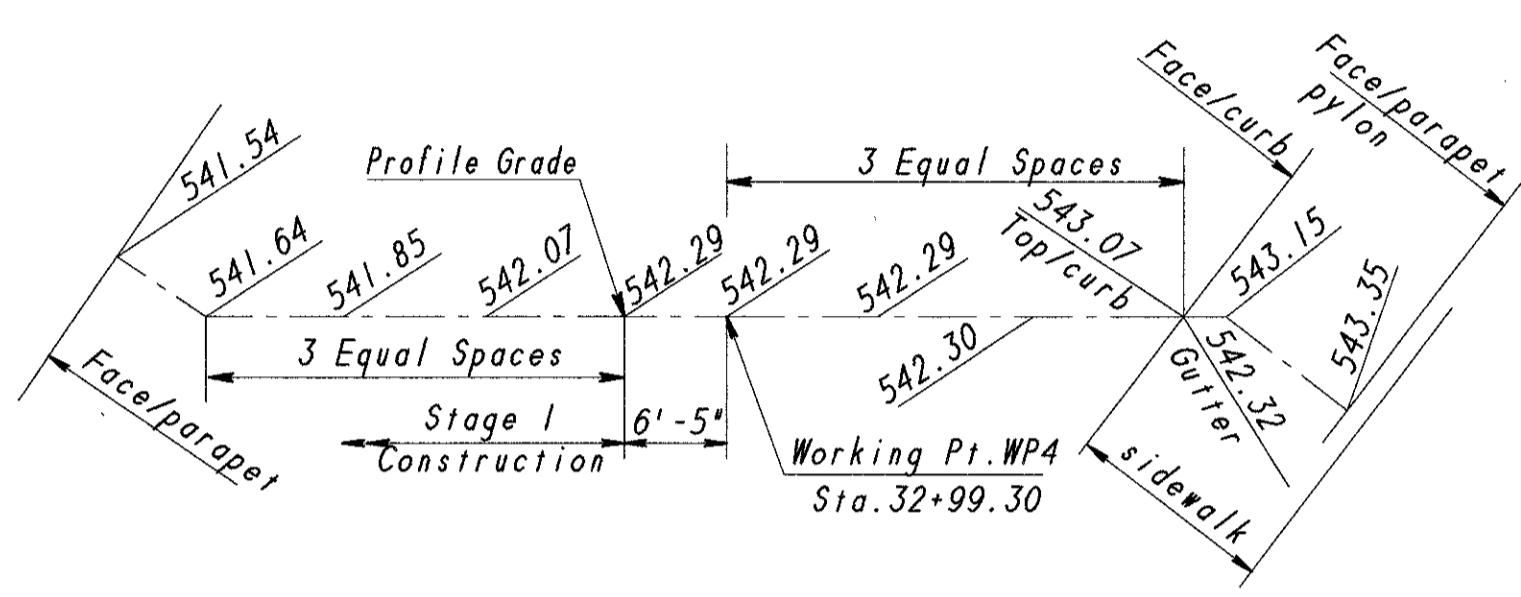
- NOTES:
1. Work the expansion joint plan at pier No.3 with Deck Slab Unit B, Sh. 61/108 with Deck Slab Unit C, Sh. 62/108 and with Ramp A Off Columbia Viaduct Sh. 74/108
 2. For Sections N-N, N'-N', R-R, R'-R', And Additional Notes See Sh. 53/108
 3. At "T" intersections and at directional change the structural steel armor shall be shop welded with full penetration groove weld. The steel retainer shall also be shop welded and the strip seal gland shall be shop vulcanized. The fabricator shall indicate on the shop drawings the location and method of field splices. The neoprene seal shall be fabricated and installed in a single continuous piece for the full length of joint after both stages of deck joint armor installation is complete. Seal installation shall begin at points of directional change and intersections.

Pflum, Klausmeyer & Gehrmann Consultants		54/108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER No.3 AND RAMP A		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	ED/PLF	WDD
DATE	REVIEWED	DATE
	IEH April 96	

JOINT/BC Scale 4



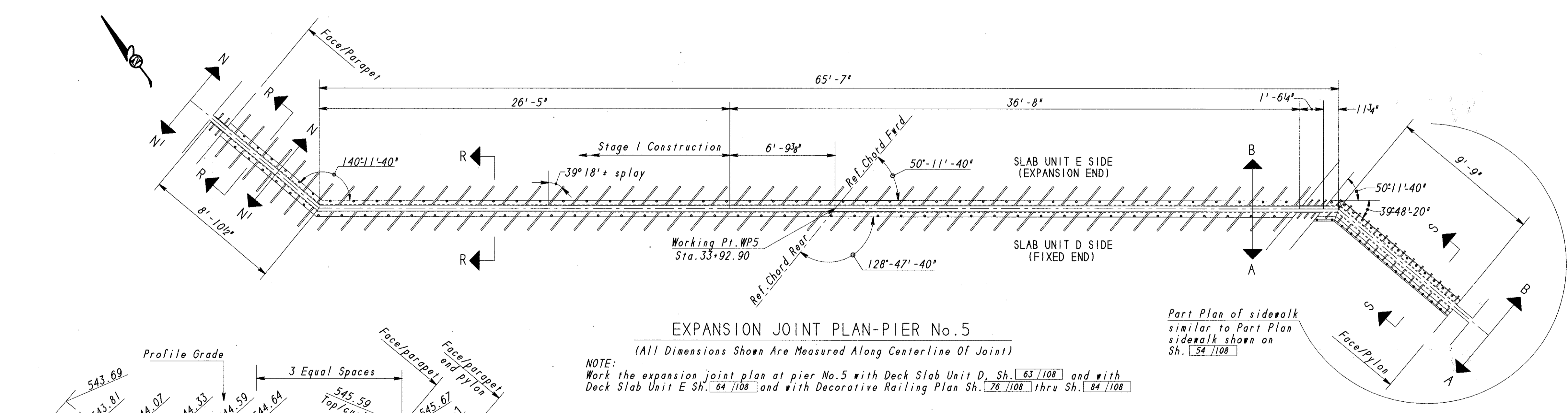
- NOTES:
1. - At directional change, the strip seal gland shall be shop vulcanized, and the steel retainer shall be shop welded.
 2. - For Sections N-N', N'-N', and R-R, see Sh. 53/108. For Sections A-A, B-B, and S-S, see Sh. 54/108.
 3. - See additional notes on sheets 53/108 and 54/108.



GRADE ELEVATIONS AT TOP OF JOINT ARMOR-PIER No. 4
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS ALONG CENTERLINE OF JOINT)

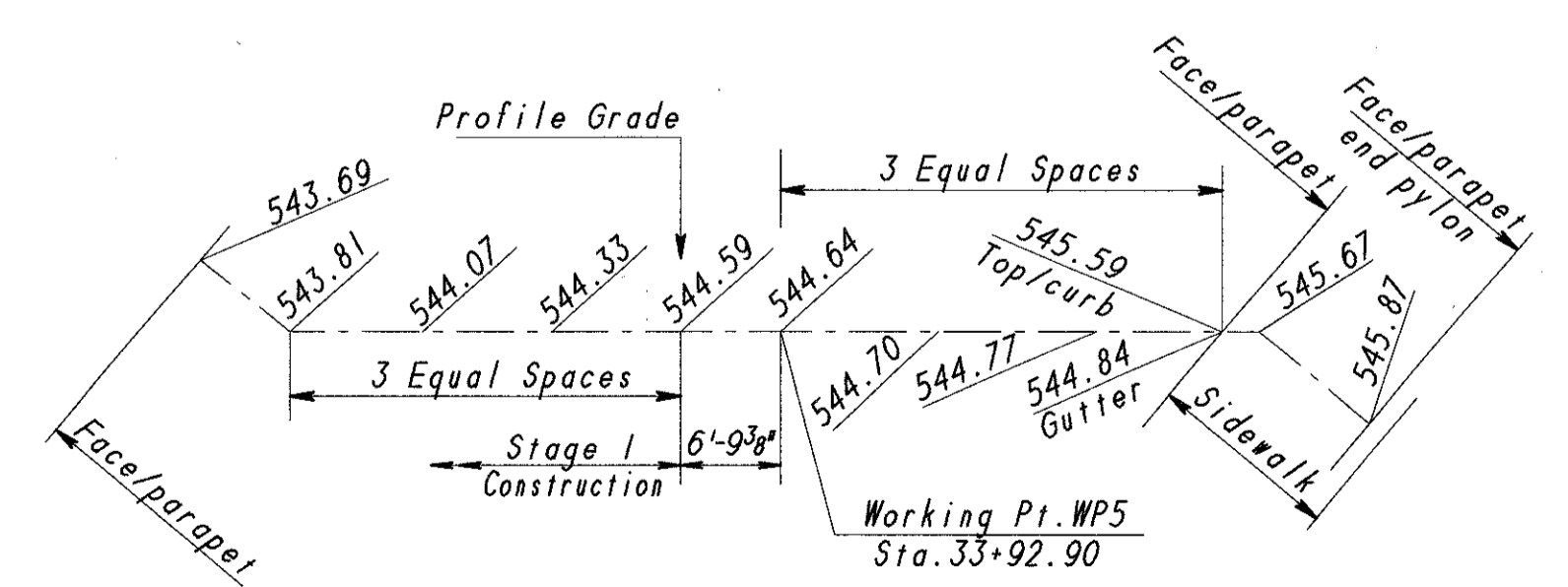
EXPANSION JOINT PLAN-PIER No. 4
(All Dimensions Shown Are Measured Along Centerline Of Joint)

NOTE:
Work the expansion joint plan at pier No. 4 with Deck Slab Unit C, Sh. 62/108 and with Deck Slab Unit D Sh. 63/108 and with Decorative Railing Plan Sh. 76/108 thru Sh. 84/108



EXPANSION JOINT PLAN-PIER No. 5
(All Dimensions Shown Are Measured Along Centerline Of Joint)

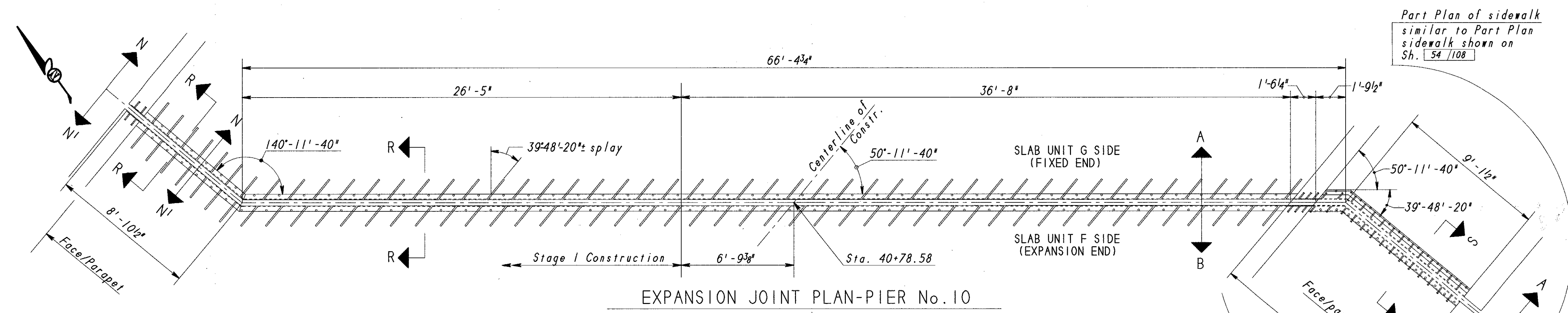
NOTE:
Work the expansion joint plan at pier No. 5 with Deck Slab Unit D, Sh. 63/108 and with Deck Slab Unit E Sh. 64/108 and with Decorative Railing Plan Sh. 76/108 thru Sh. 84/108



GRADE ELEVATIONS AT TOP OF JOINT ARMOR-PIER No. 5
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS ALONG CENTERLINE OF JOINT)

PK&G Prill, Klausmeyer & Gehrmann Consultants		55 / 108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT PIER No. 4 AND PIER No. 5		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY DESIGNED: ED DRAWN: ED/PLF TRACED: WDD CHECKED: WDD REVIEWED: IEH DATE: April 96		Sta. 30+55.40 Sta. 47+16.19 REVISED:

JOINTCD Scale 4



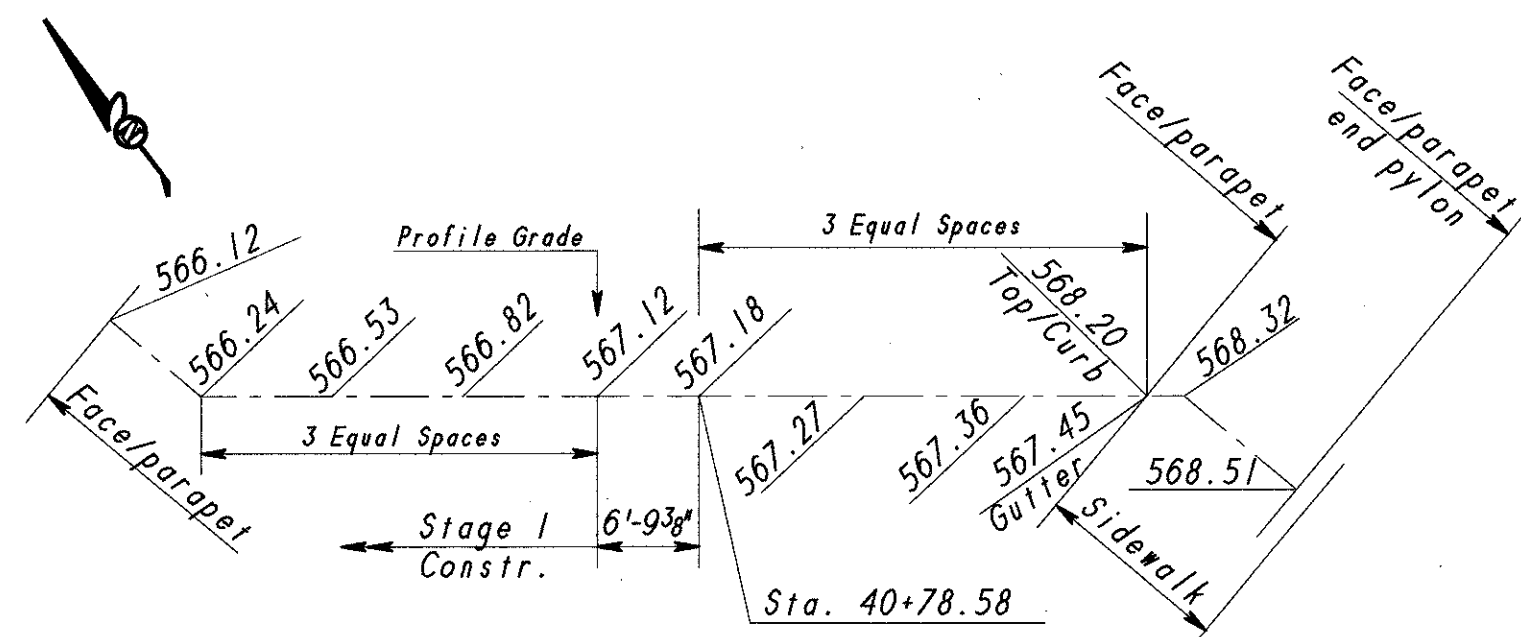
EXPANSION JOINT PLAN-PIER No. 10

(All Dimensions Shown Are Measured Along Centerline Of Joint)

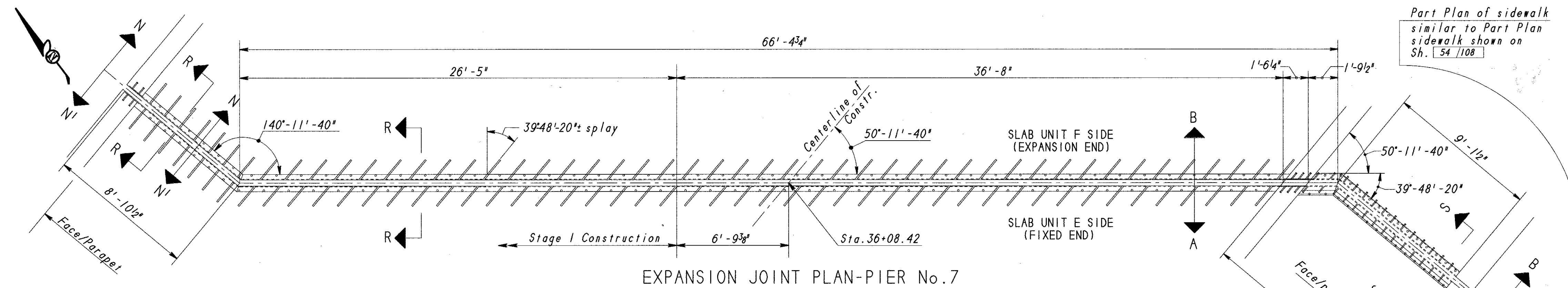
NOTE: Work the expansion joint plan at piers noted with Deck Slab Unit F, Sh. 66/108 and with Deck Slab Unit G, Sh. 67/108 and with Decorative Railing Plan, Sh. 76/108 thru Sh. 84/108

NOTES:

- At directional change, the strip seal gland shall be shop vulcanized, and the steel retainer shall be shop welded.
- For Sections N-N, N'-N', and R-R, see Sh. 53/108. For Sections A-A, B-B, and S-S, see Sh. 54/108.
- See additional notes on sheets 53/108 and 54/108.



GRADE ELEVATIONS AT TOP OF JOINT ARMOR-PIER No. 10
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS ALONG CENTERLINE OF JOINT)

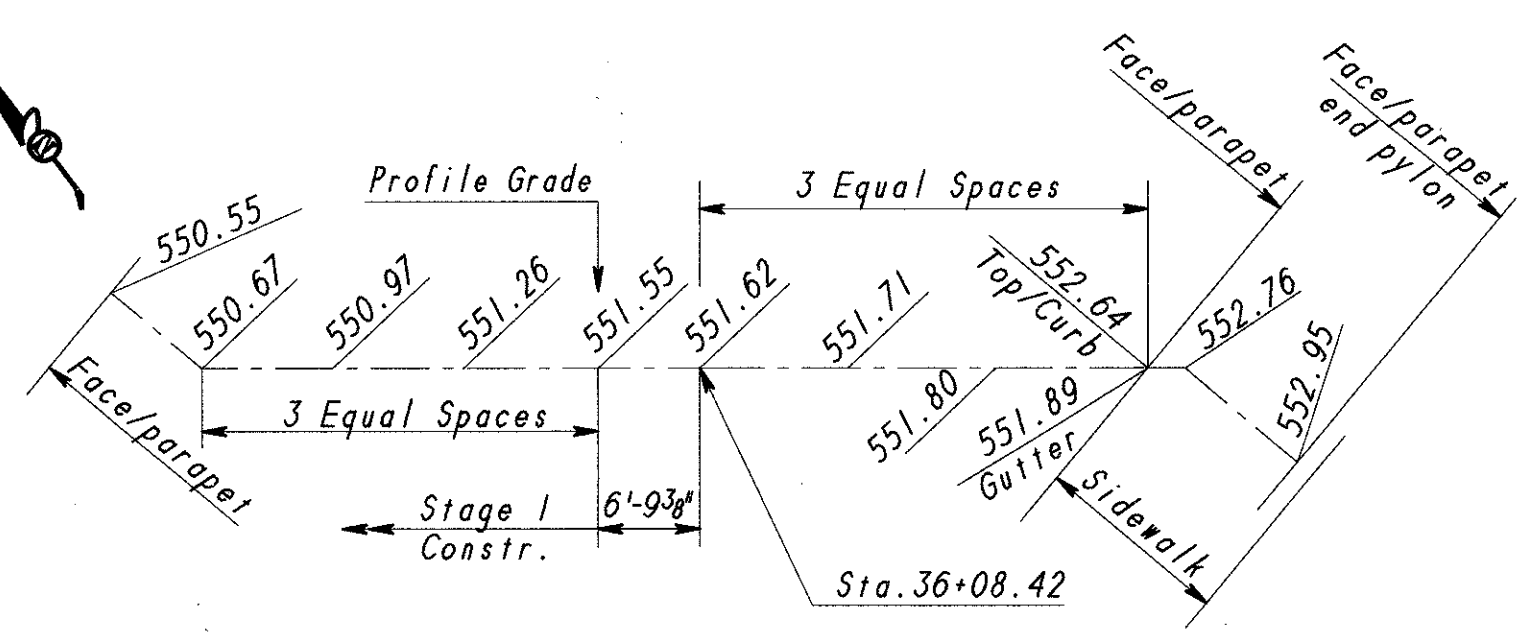


EXPANSION JOINT PLAN-PIER No. 7

(All Dimensions Shown Are Measured Along Centerline Of Joint)

NOTE: Work the expansion joint plan at piers noted with Deck Slab Unit E, Sh. 64/108 and with Deck Slab Unit F, Sh. 65/108 and with Decorative Railing Plan, Sh. 76/108 thru Sh. 84/108

NOTE: For Side View N-N and N'-N' see Sh. 55/108

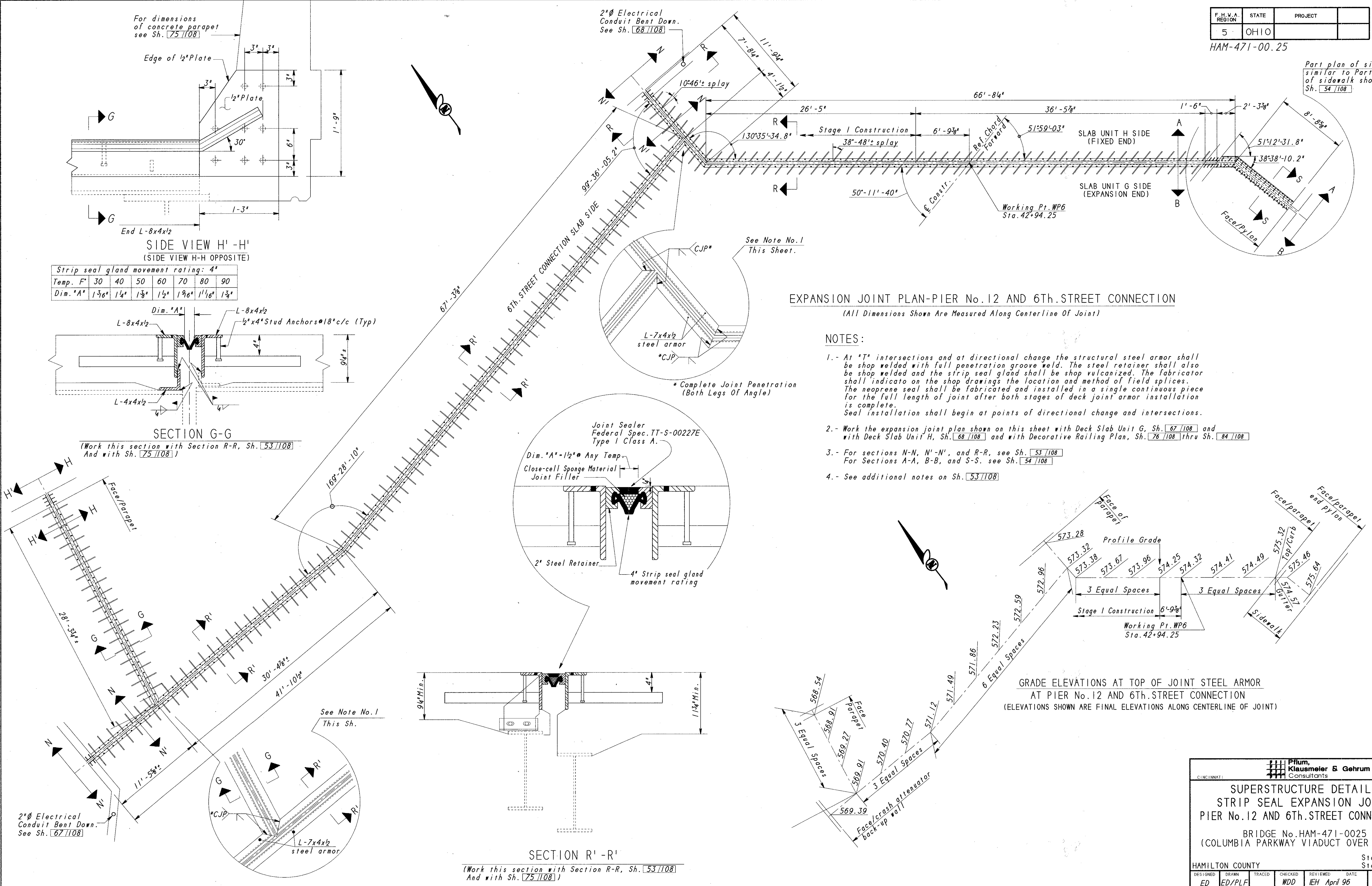


GRADE ELEVATIONS AT TOP OF JOINT ARMOR-PIER No. 7
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS ALONG CENTERLINE OF JOINT)

CINCINNATI		Plum, Klausmeyer & Gehrum Consultants		56/108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT PIER No. 7 AND PIER No. 10				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
				Sta. 30+55.40
				Sta. 47+16.19
HAMILTON COUNTY				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
ED	ED/PLF		WDD	IEH April 96

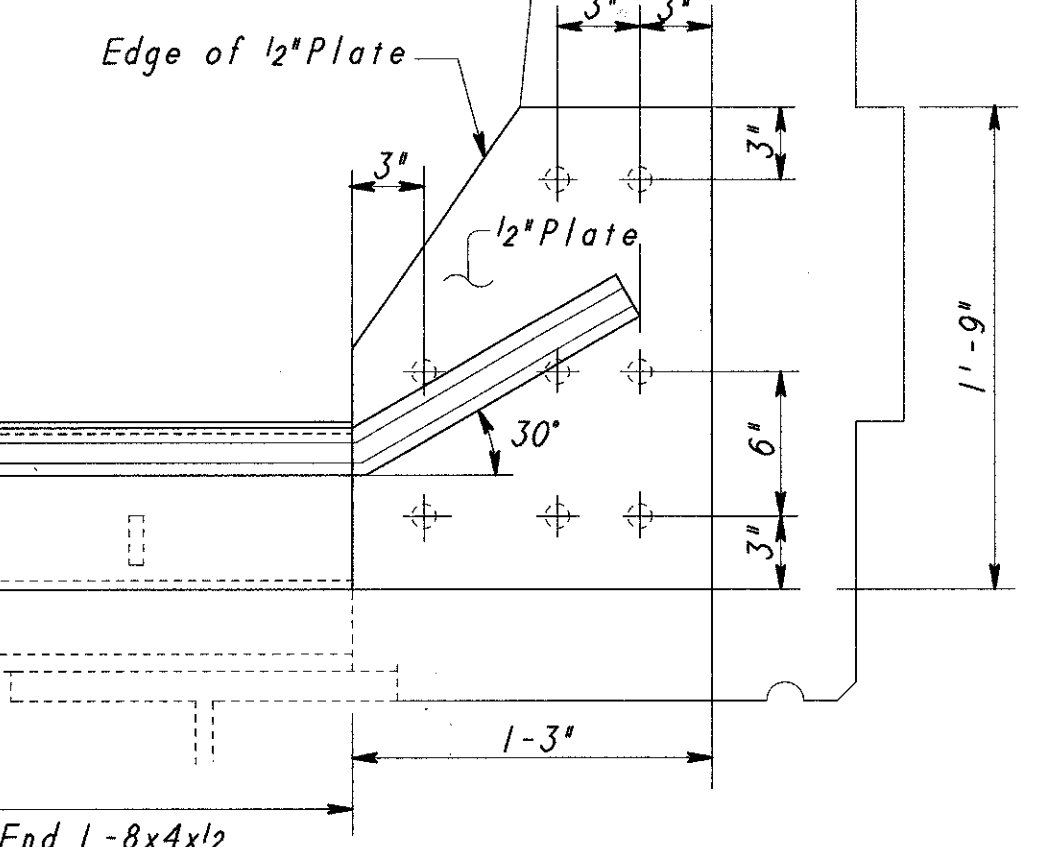
JOINTS Scale 4

Part plan of sidewalk similar to Part Plan of sidewalk shown on Sh. 54/108



For dimensions of concrete parapet see Sh. 75/108

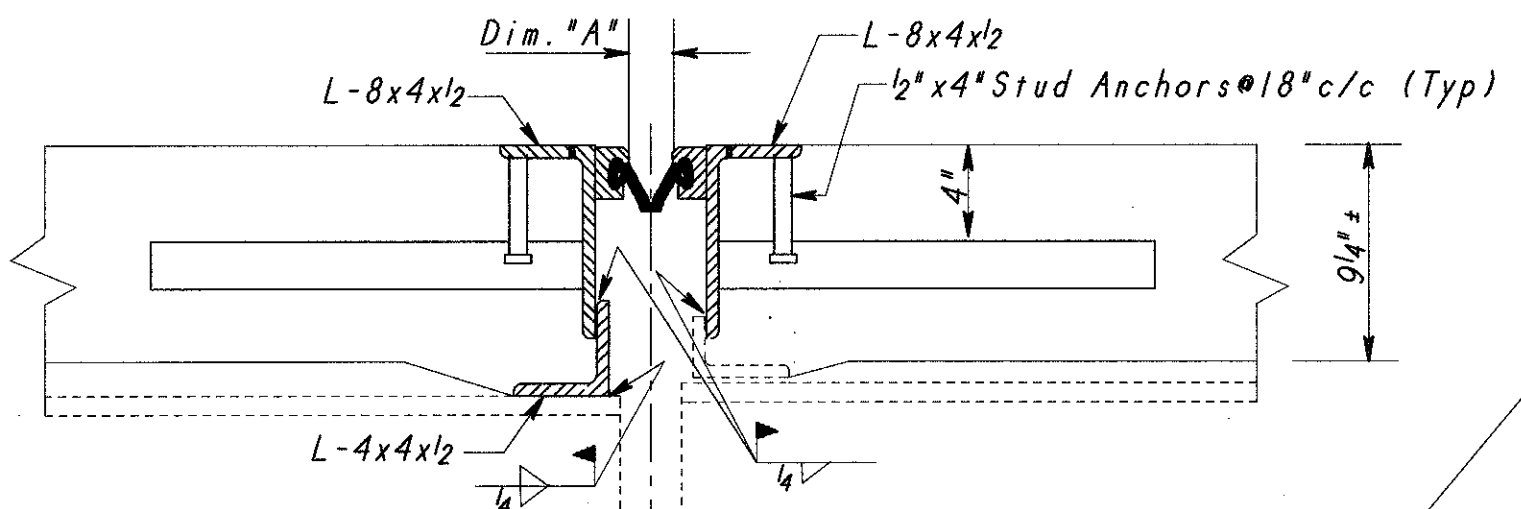
2"Ø Electrical Conduit Bent Down. See Sh. 68/108



SIDE VIEW H'-H' (SIDE VIEW H-H OPPOSITE)

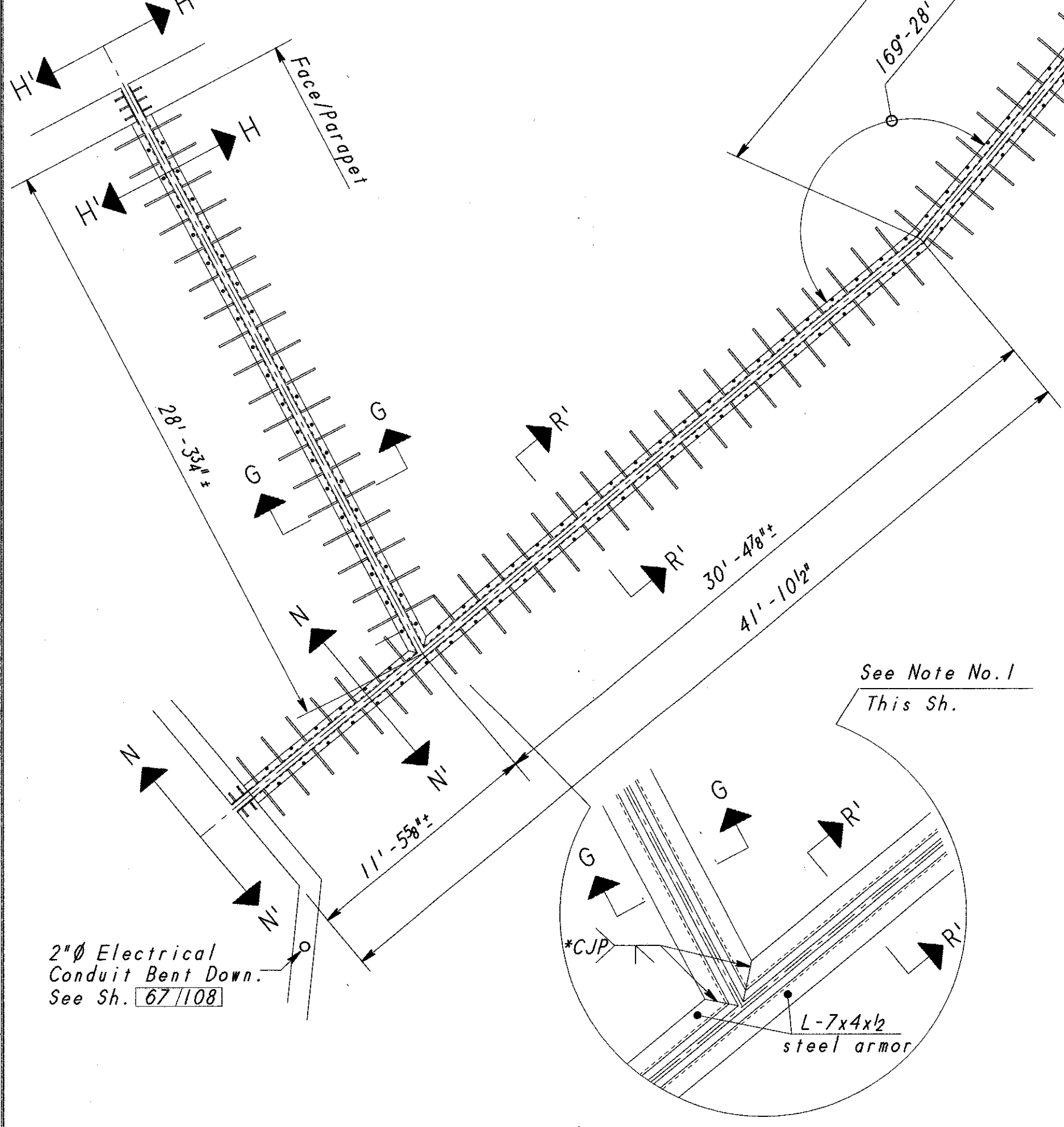
Strip seal gland movement rating: 4"

Temp. F°	30	40	50	60	70	80	90
Dim. "A"	1 3/16"	1 1/4"	1 3/8"	1 1/2"	1 9/16"	1 11/16"	1 3/4"



SECTION G-G

(Work this section with Section R-R, Sh. 53/108 And with Sh. 75/108)



SECTION R'-R'

(Work this section with Section R-R, Sh. 53/108 And with Sh. 75/108)

EXPANSION JOINT PLAN-PIER No. 12 AND 6TH STREET CONNECTION

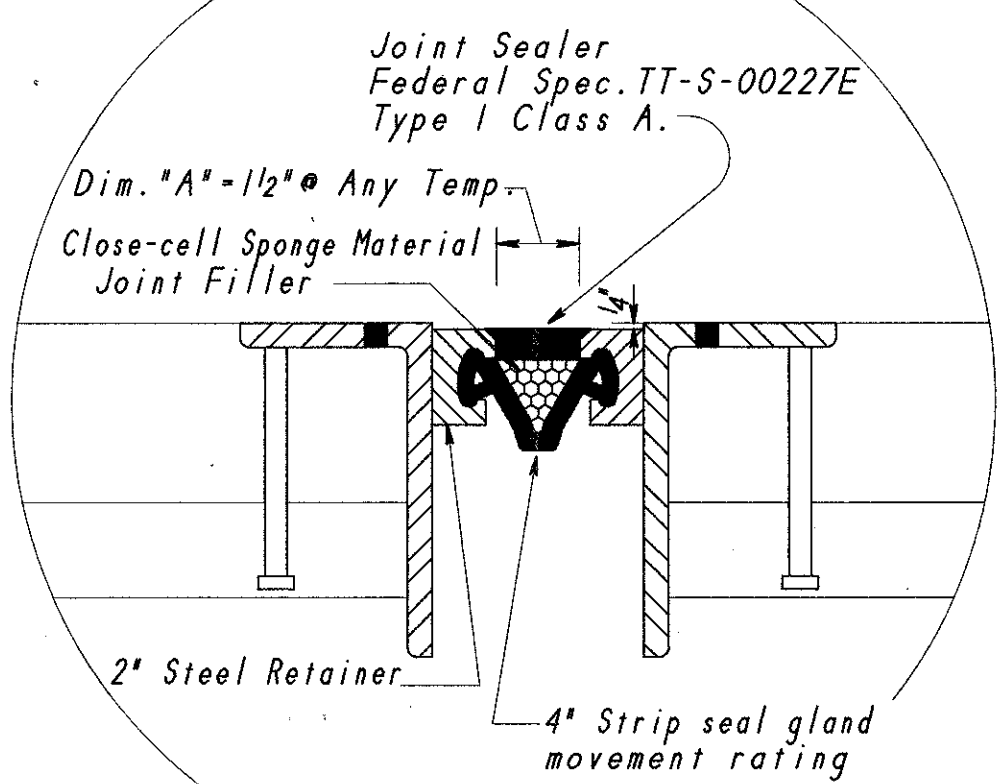
(All Dimensions Shown Are Measured Along Centerline Of Joint)

NOTES:

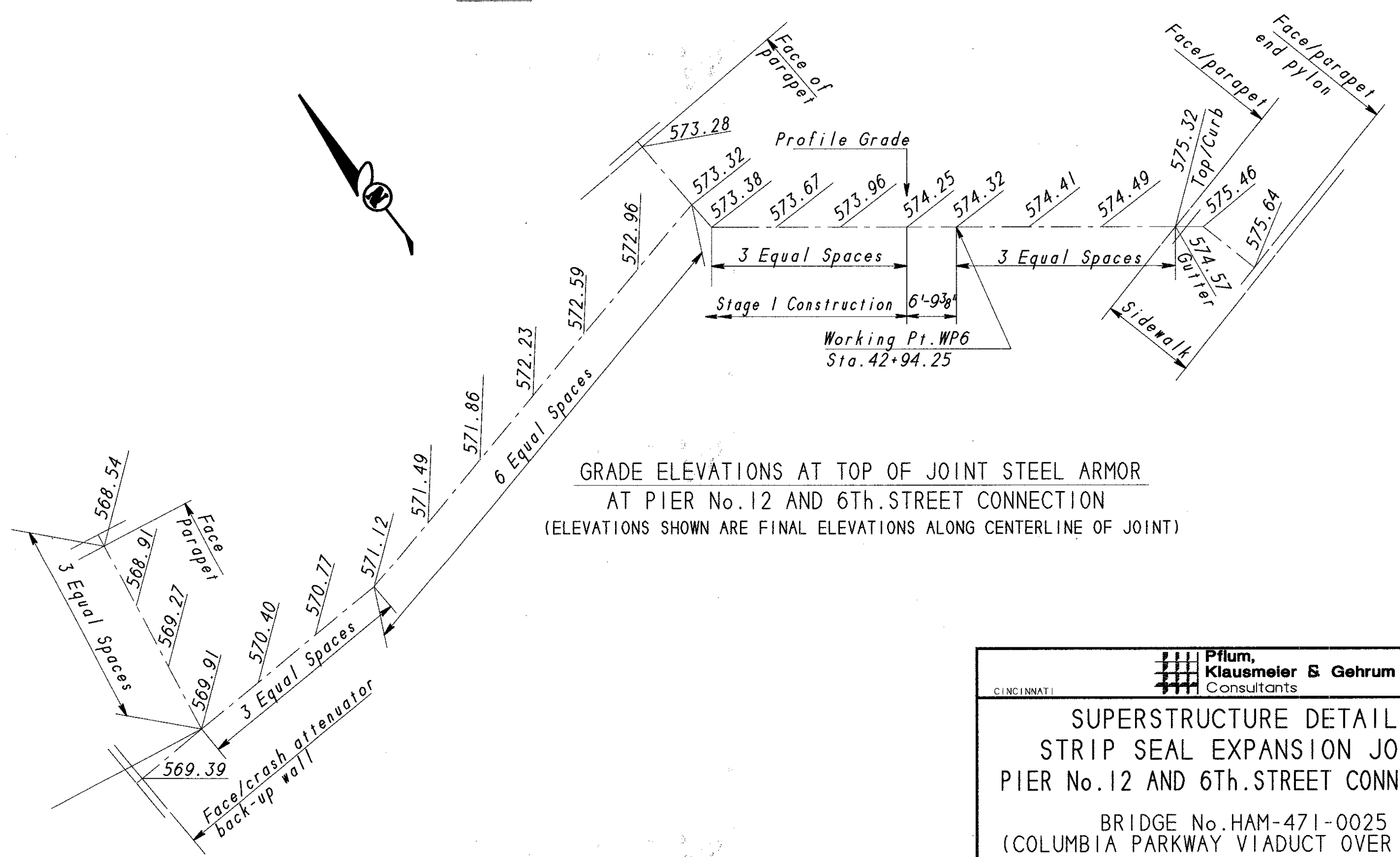
1. At "T" intersections and at directional change the structural steel armor shall be shop welded with full penetration groove weld. The steel retainer shall also be shop welded and the strip seal gland shall be shop vulcanized. The fabricator shall indicate on the shop drawings the location and method of field splices. The neoprene seal shall be fabricated and installed in a single continuous piece for the full length of joint after both stages of deck joint armor installation is complete. Seal installation shall begin at points of directional change and intersections.
2. Work the expansion joint plan shown on this sheet with Deck Slab Unit G, Sh. 67/108 and with Deck Slab Unit H, Sh. 68/108 and with Decorative Railing Plan, Sh. 76/108 thru Sh. 84/108
3. For sections N-N, N'-N', and R-R, see Sh. 53/108 For Sections A-A, B-B, and S-S, see Sh. 54/108
4. See additional notes on Sh. 53/108

* Complete Joint Penetration (Both Legs Of Angle)

Joint Sealer Federal Spec. TT-S-00227E Type I Class A.



4" Strip seal gland movement rating

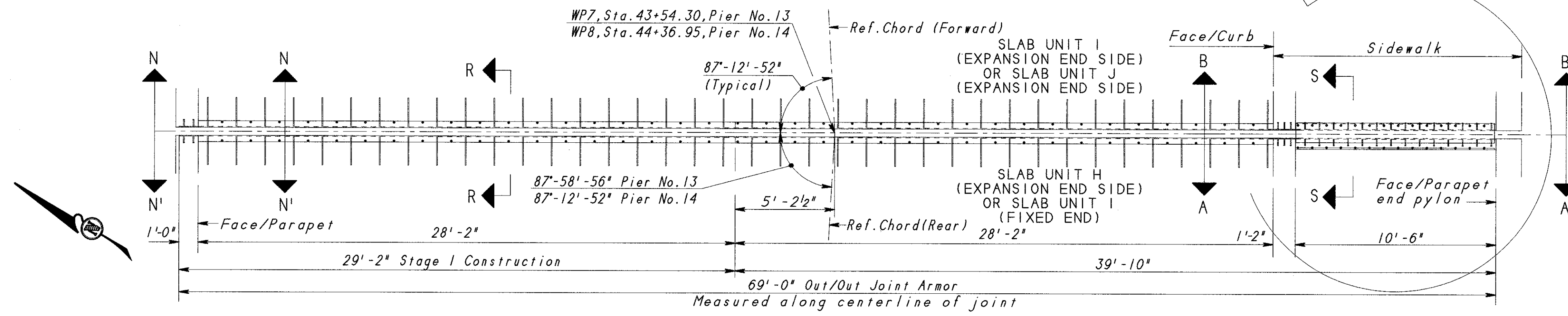


GRADE ELEVATIONS AT TOP OF JOINT STEEL ARMOR AT PIER No. 12 AND 6TH STREET CONNECTION (ELEVATIONS SHOWN ARE FINAL ELEVATIONS ALONG CENTERLINE OF JOINT)

JOINTGH Scale 5:3333

				57/108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT PIER No. 12 AND 6TH STREET CONNECTION				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY				Sta. 30+55.40
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
ED	ED/PLF	WDD	IEH	April 96
				DATE
				REVISION

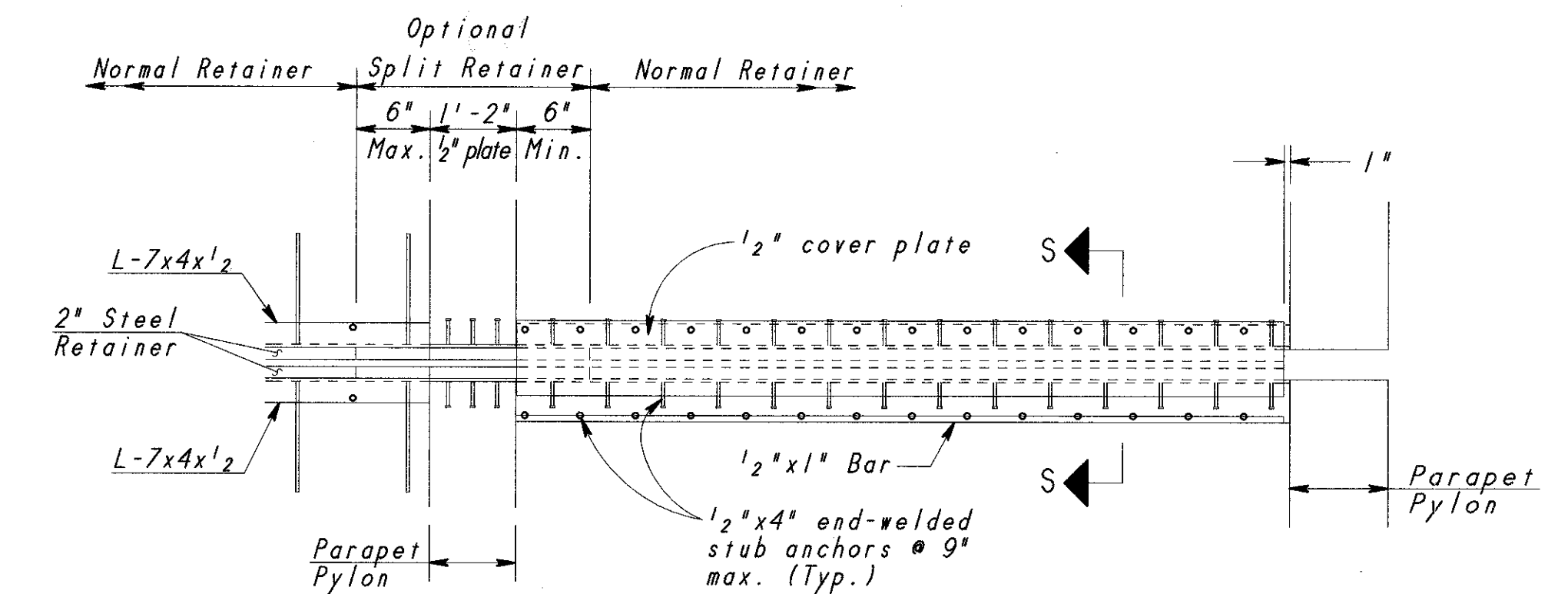
See Part Plan This Sh. (Typical)



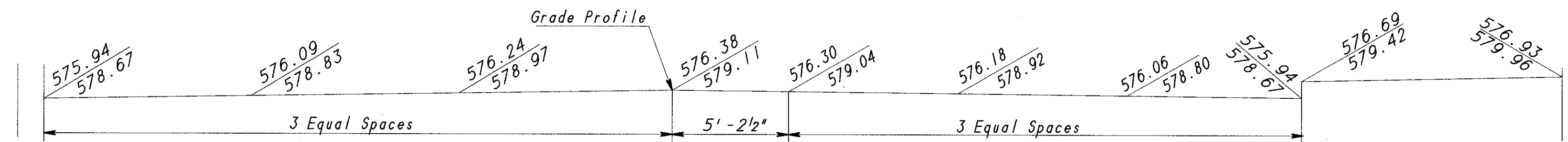
EXPANSION JOINT PLAN-PIER No. 13 & No. 14

(All Dimensions Shown Are Measured Along Centerline Of Joint)

NOTE: Work the expansion joint plan at piers noted with Deck Slab Unit H, Sh. [68/108] with Deck Slab Unit I, Sh. [69/108], with Deck Slab Unit J, Sh. [70/108], and with Decorative Railing Plan, Sh. [76/108] thru Sh. [84/108].



PART PLAN AT SIDEWALK



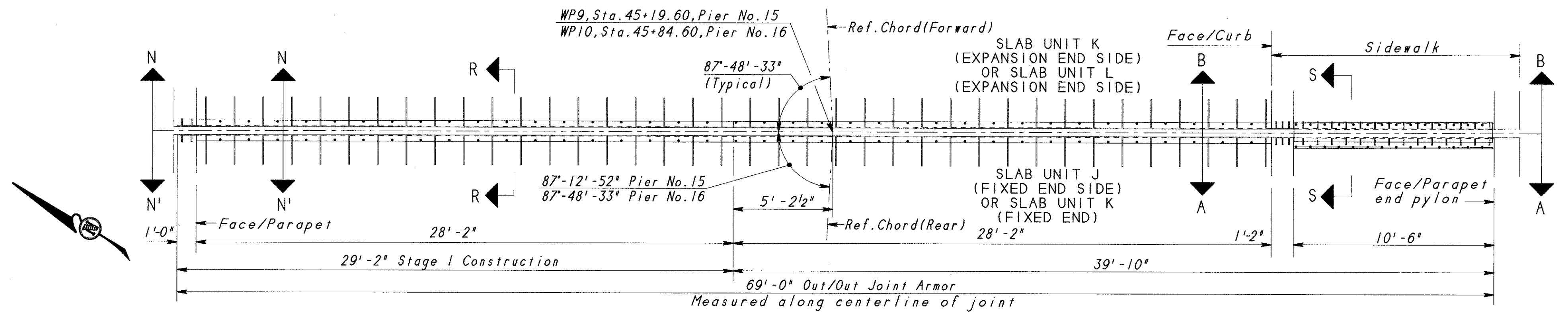
CROSS-SLOPE ELEVATIONS AT TOP OF JOINT STEEL ARMOR

PIER NO. 13
PIER NO. 14

(ELEVATIONS SHOWN ARE FINAL ELEVATIONS AT TOP OF STEEL ARMOR ALONG CENTERLINE OF JOINT)

NOTES:

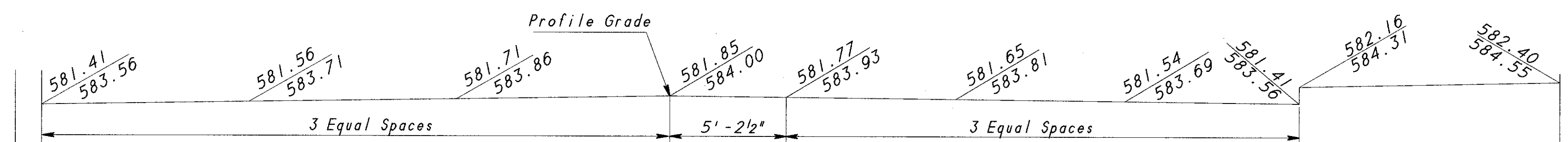
- For Sections N-N, N'-N', and R-R, see Sh. [53/108]
- For Sections A-A, B-B, and S-S, see Sh. [54/108]
- See Additional Notes On Sh. [53/108]



EXPANSION JOINT PLAN-PIER No. 15 & No. 16

(All Dimensions Shown Are Measured Along Centerline Of Joint)

NOTE: Work the expansion joint plan at piers noted with Deck Slab Unit J, Sh. [70/108] with Deck Slab Unit K, Sh. [71/108], with Deck Slab Unit L, Sh. [72/108], and with Decorative Railing Plan, Sh. [76/108] thru Sh. [84/108].



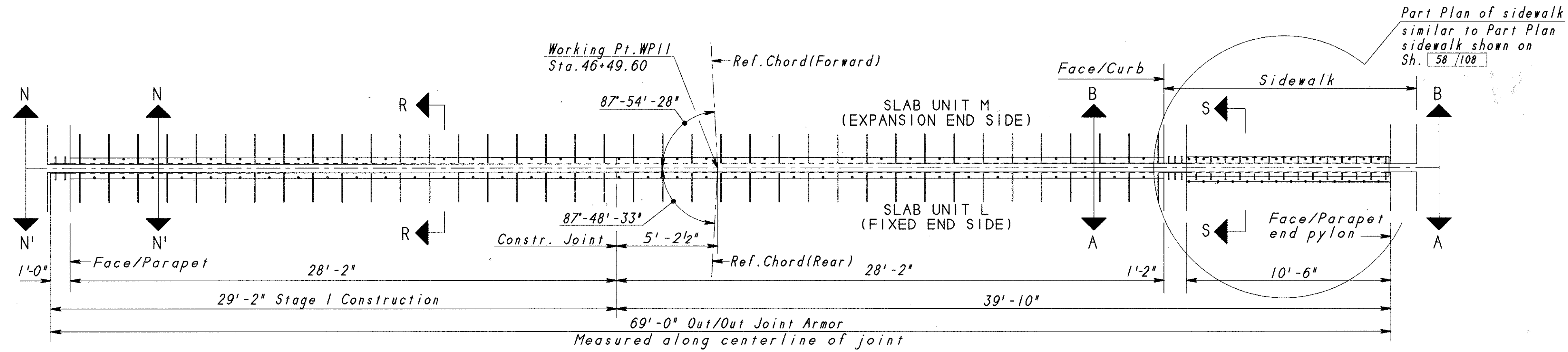
CROSS-SLOPE ELEVATIONS AT TOP OF JOINT STEEL ARMOR

PIER NO. 15
PIER NO. 16

(ELEVATIONS SHOWN ARE FINAL ELEVATIONS AT TOP OF STEEL ARMOR ALONG CENTERLINE OF JOINT)

Plum, Klausmeier & Gehrum Consultants		58/108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER NO. 13 THRU 16		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96
		REVISED

JT13-16 Scale 4



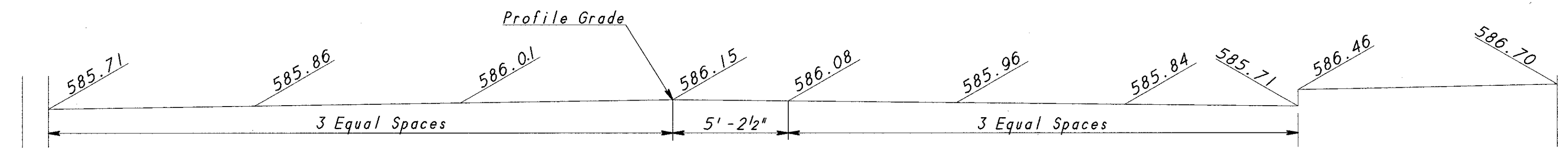
EXPANSION JOINT PLAN-PIER No. 17

(All Dimensions Shown Are Measured Along Centerline Of Joint)

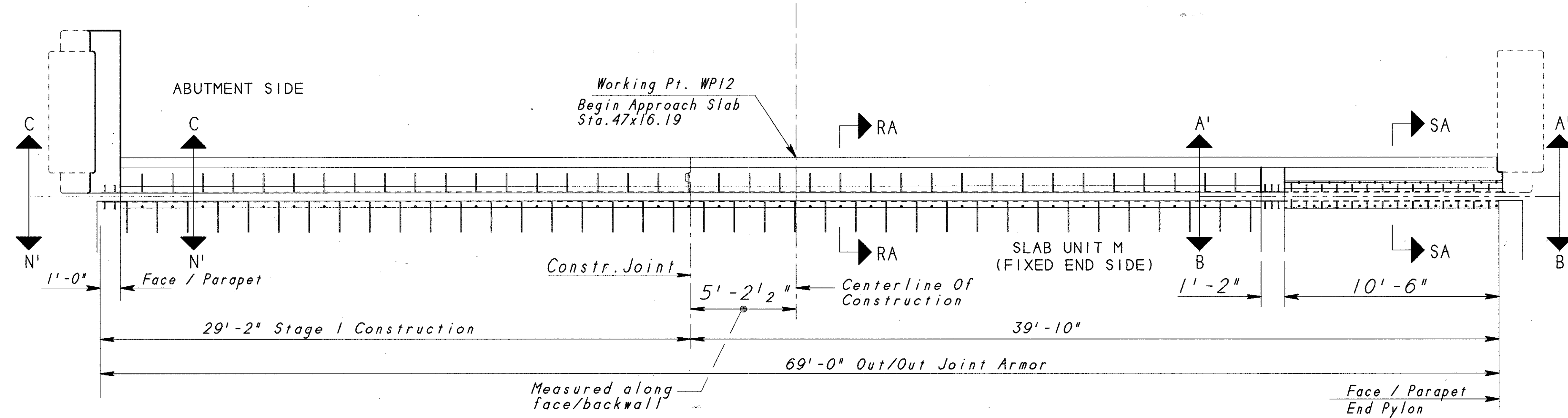
NOTE:
Work the expansion joint plan at piers noted with Deck Slab Unit L, Sh. 72/108 and with Deck Slab Unit M, Sh. 73/108 and with Decorative Railing Plan, Sh. 76/108 thru Sh. 84/108

NOTES:

- 1.- For Sections A'-A', C-C, N-N, N'-N', R-R, RA-RA, and SA-SA, see Sh. 53/108
- 2.- For Sections A-A, B-B, and S-S, see Sh. 54/108
- 3.- See Additional Notes On Sh. 53/108

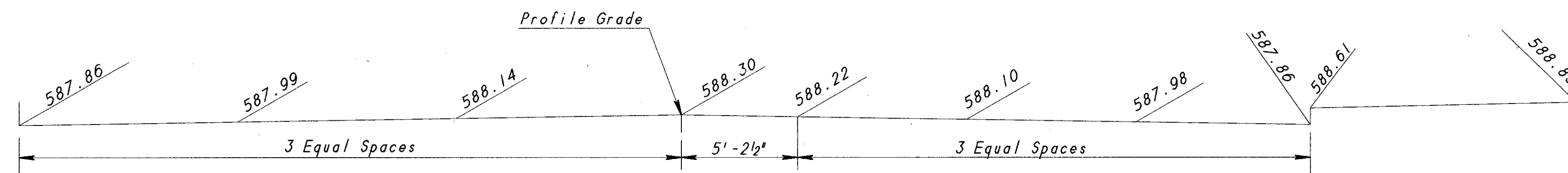


CROSS-SLOPE ELEVATIONS AT TOP OF JOINT STEEL ARMOR PIER NO. 17
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS AT TOP OF STEEL ARMOR ALONG CENTERLINE OF JOINT)



JOINT PLAN-FORWARD ABUTMENT

NOTE:
Work the joint plan at forward abutment with FORWARD ABUTMENT (Abutment No. 18), Sh. 16/108 and with Deck Slab Unit M, Sh. 73/108 and with Decorative Railing Plan, Sh. 76/108 thru Sh. 84/108



CROSS-SLOPE ELEVATIONS AT TOP OF JOINT STEEL ARMOR-FORWARD ABUTMENT
(ELEVATIONS SHOWN ARE FINAL ELEVATIONS AT TOP OF STEEL ARMOR ALONG FACE/BACKWALL)

Pflum, Klausmeier & Gehrum Consultants		59/108
SUPERSTRUCTURE DETAILS STRIP SEAL EXPANSION JOINT AT PIER NO. 17 AND AT FORWARD ABUTMENT (NO. 18) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471) Sta. 30+55.40 Sta. 47+16.19		
HAMILTON COUNTY		
DESIGNED	DRAWN	TRACED
ED	ED/PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

JT17-FA Scale 4

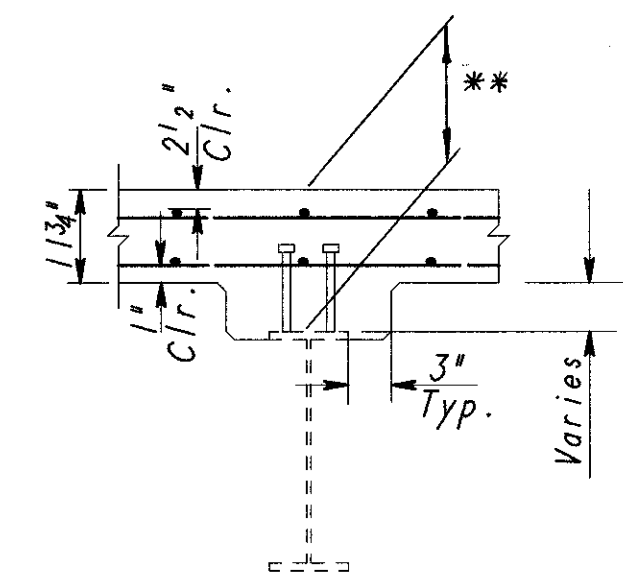
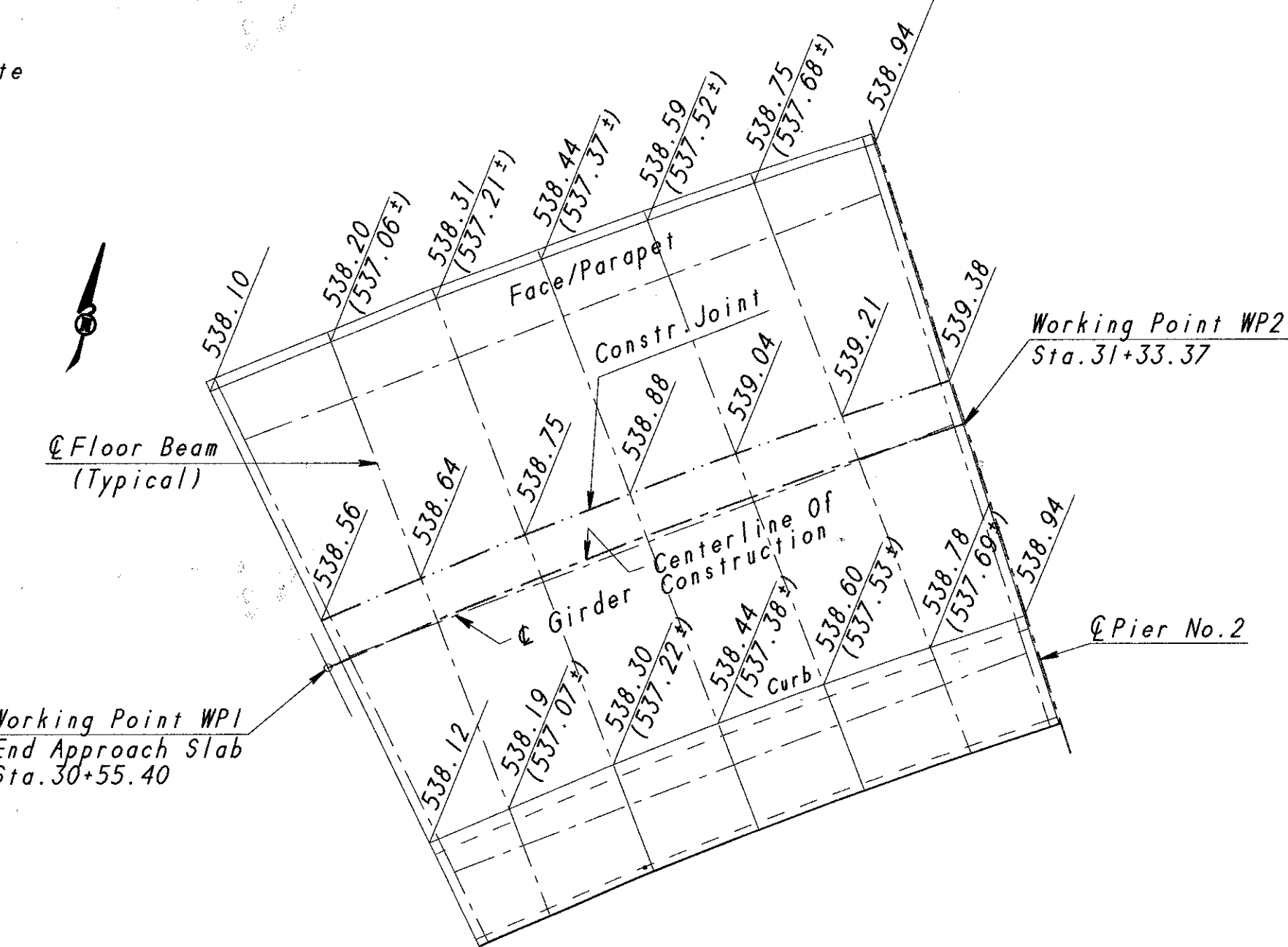
HAM-471-00.25

LEGEND

Existing Structure
Proposed Work

NOTES

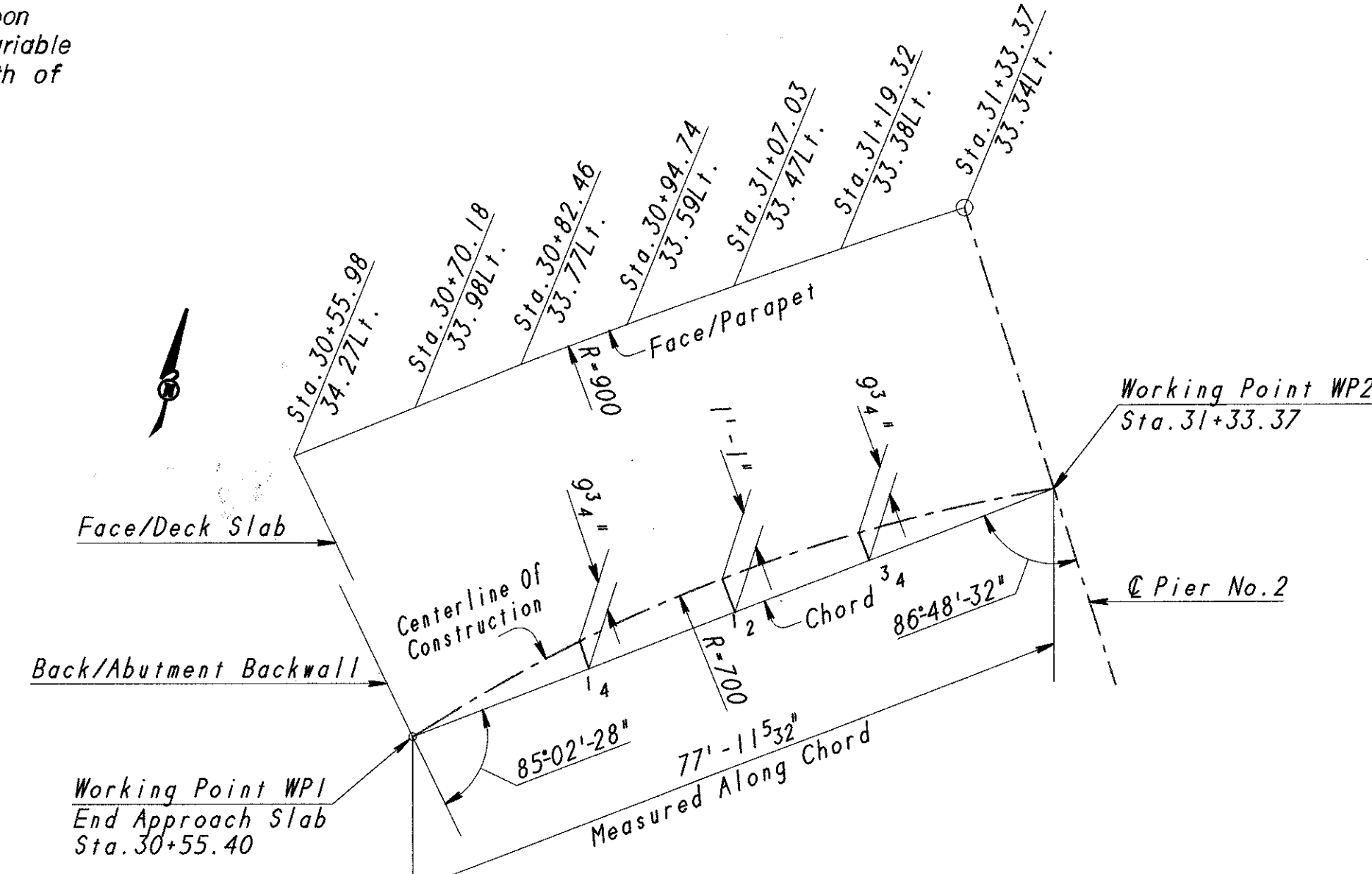
- Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.
- Work This Sheet With General Plan Sh. 4/108.
- See REINFORCING STEEL BAR LIST, Sh. 88/108.
- See MISCELLANEOUS DETAILS, Sh. 12/108.
- See GENERAL NOTES, Sh. 7/108 Thru 11/108.
- Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 77/108.



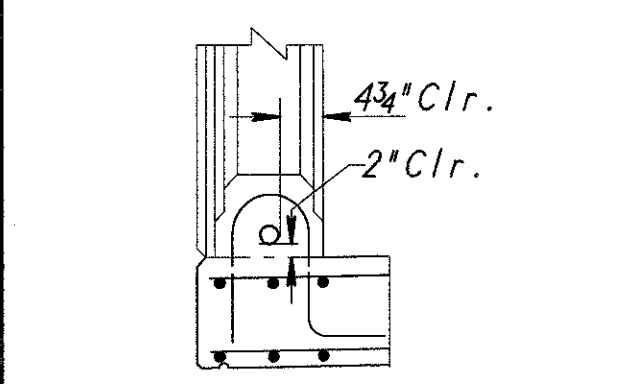
SECTION THRU FLOOR BEAM (TYPICAL)

** This is the design dimension. The quantity of concrete to be paid for will be based upon this dimension taking into account the variable haunch depth plus the nominal slab depth of 11-3/4".

LAP LENGTH TABLE	
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

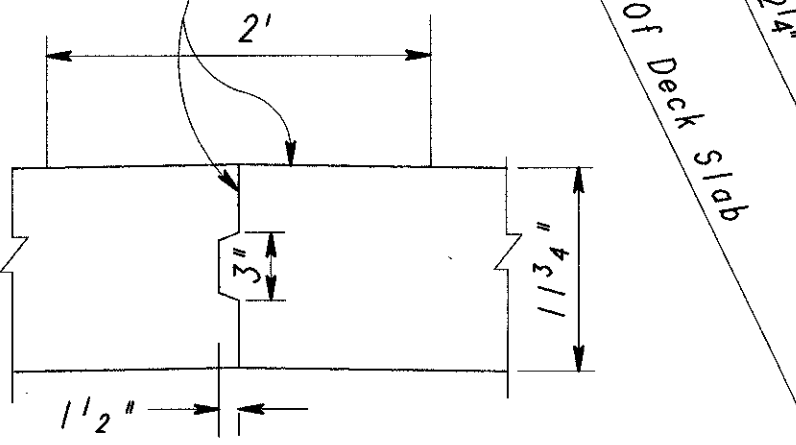


DECK SPAN SCHEMATIC LAYOUT PLAN

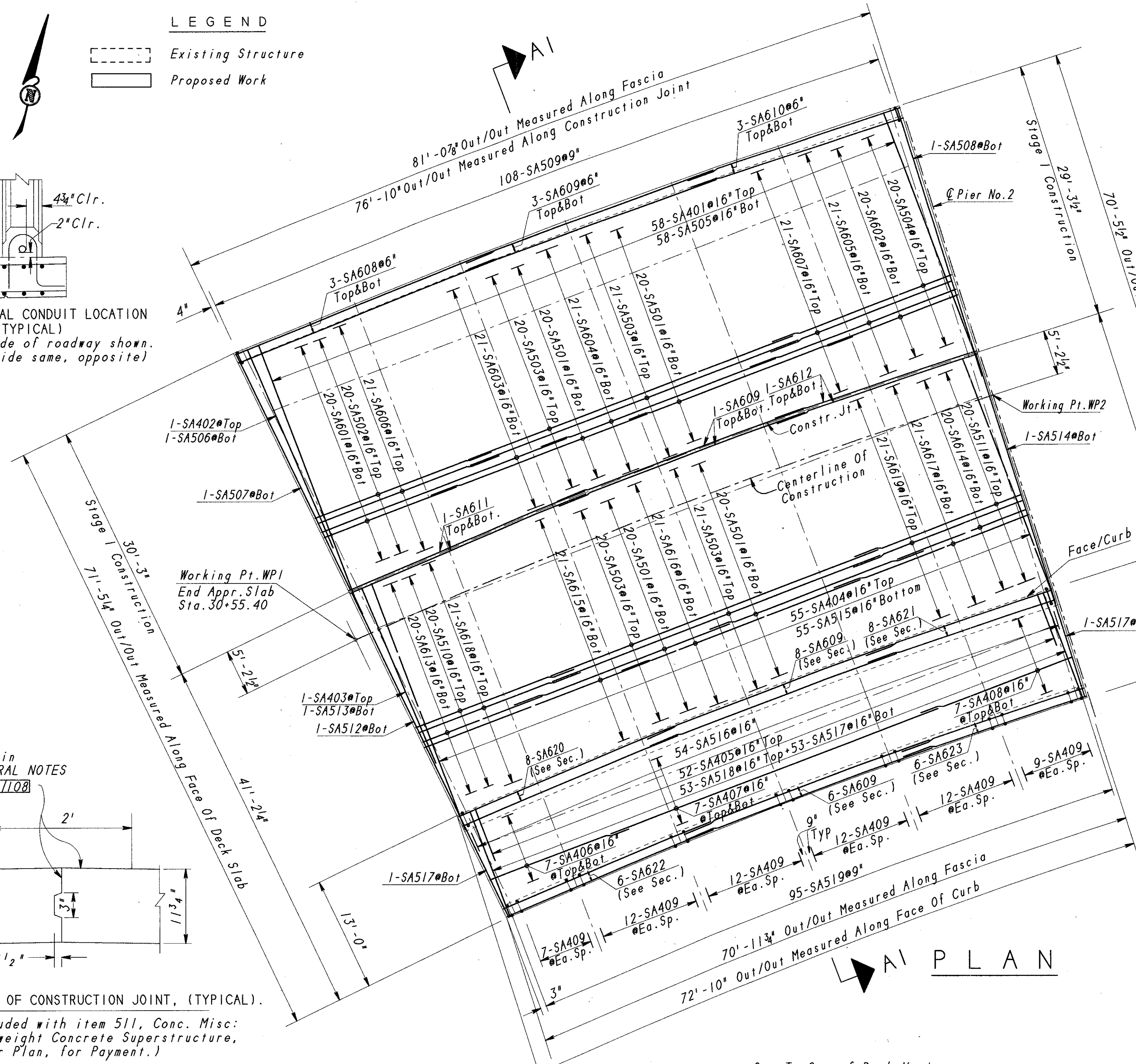


ELECTRICAL CONDUIT LOCATION (TYPICAL)
(Left side of roadway shown. Right side same, opposite)

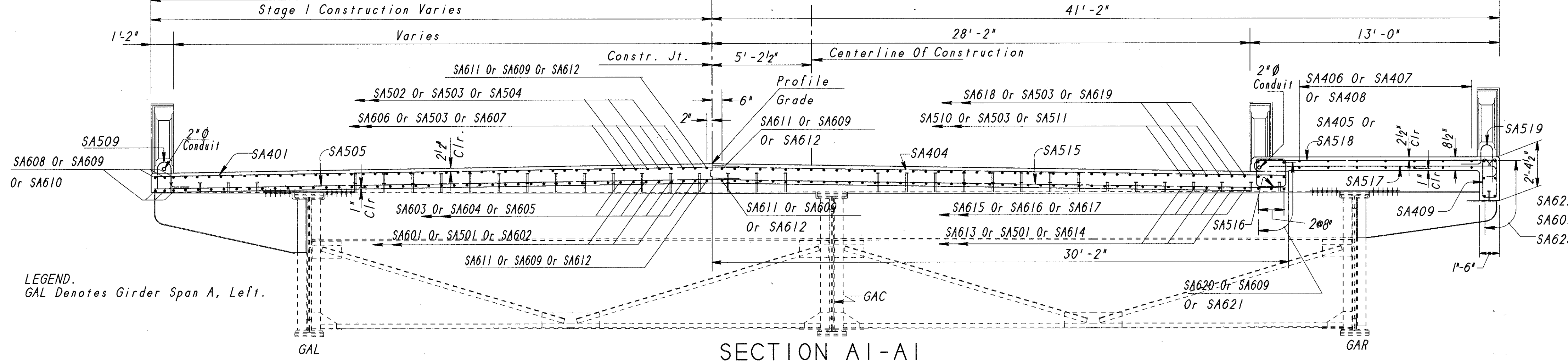
HMWM Resin
See GENERAL NOTES
Sheet 11/108



SEALING OF CONSTRUCTION JOINT (TYPICAL).
(Included with item 511, Conc. Misc. Lightweight Concrete Superstructure, As per Plan, for Payment.)



Out To Out of Deck Varies



LEGEND.
GAL Denotes Girder Span A, Left.

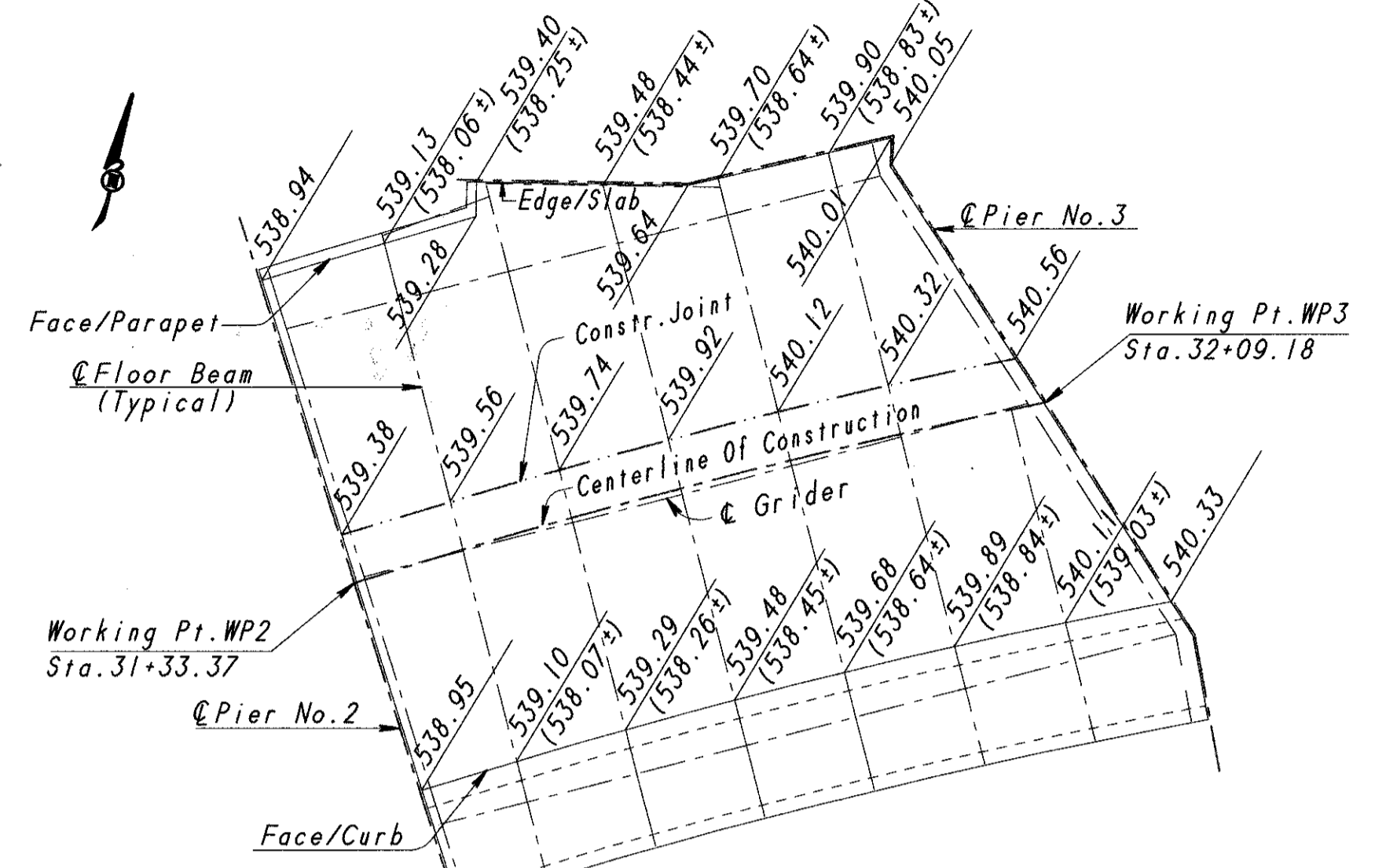
Plum, Klausmeyer & Gehrm Consultants		60 / 108
SPAN 1-2 DECK SLAB UNIT A		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	ED	WDD
REVIEWED	DATE	REVISED
IEH	April 96	

File SPN12 Scale 8

HAM-471-00.25

NOTE -
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fascia, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

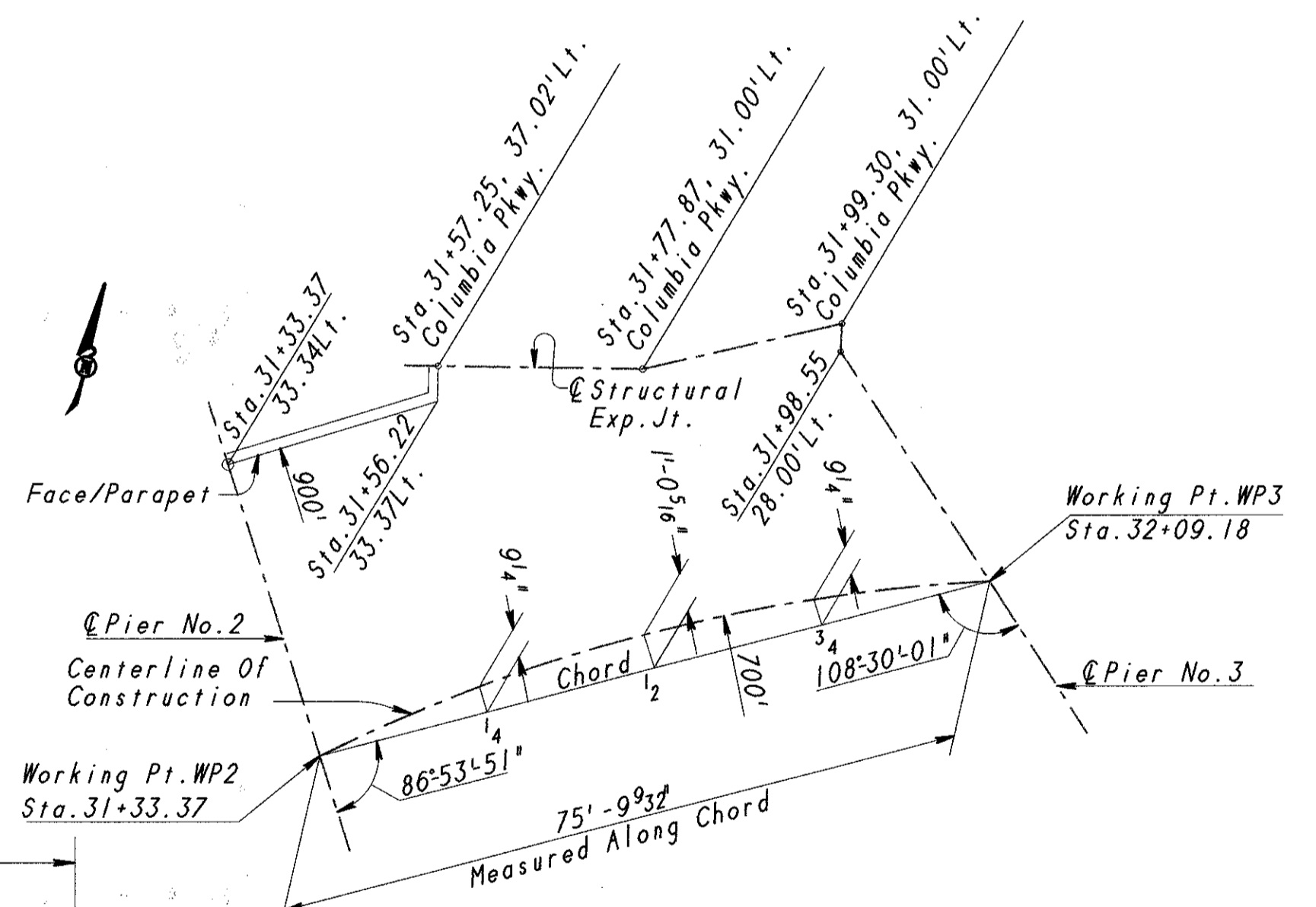
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"



SCREED ELEVATIONS SCHEMATIC DIAGRAM

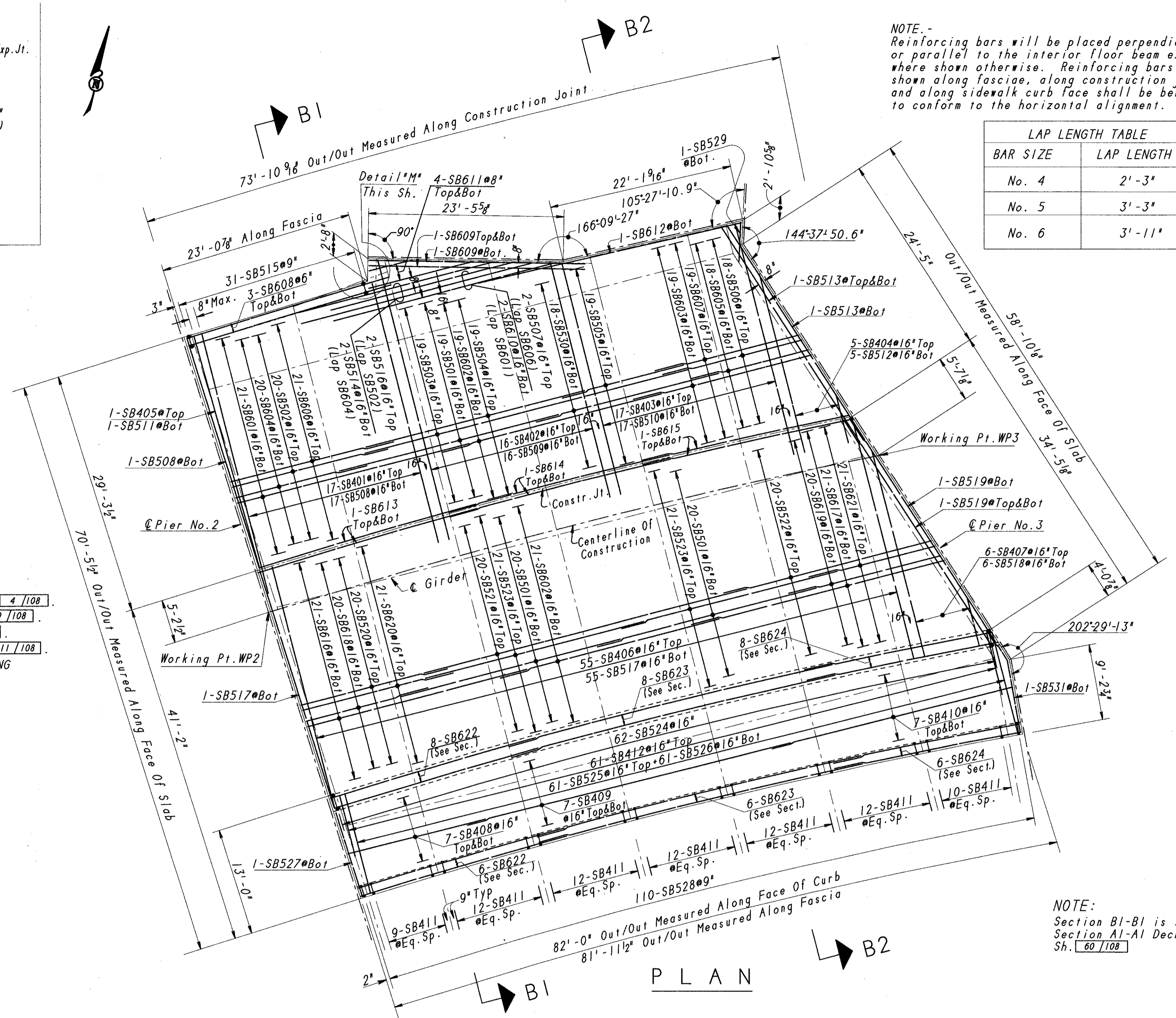
(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND
123.45.....Denotes Top Of Slab Elevation.
(123.45±).....Denotes Top Of Existing Steel Elevation.

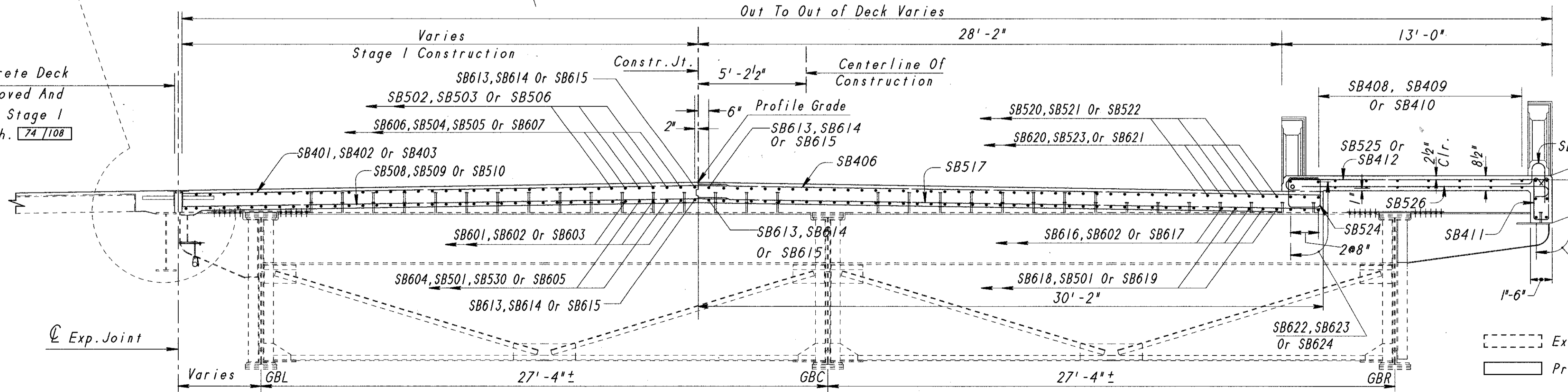


DECK SPAN SCHEMATIC LAYOUT PLAN

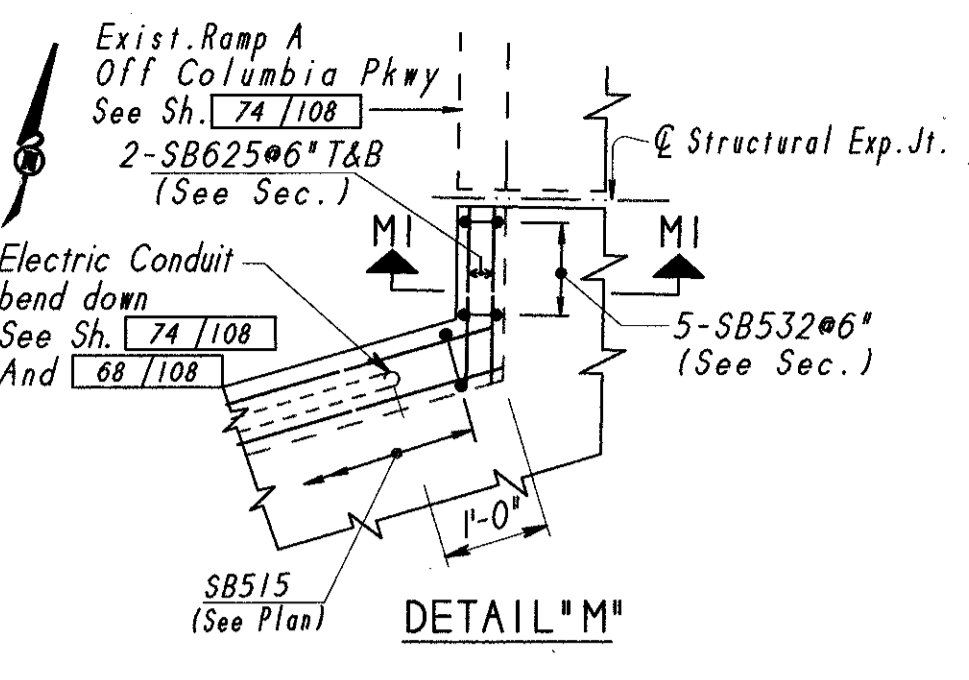
NOTE:
Section B1-B1 is similar to Section A1-A1 Deck Span 1-2 Sh. 60/108



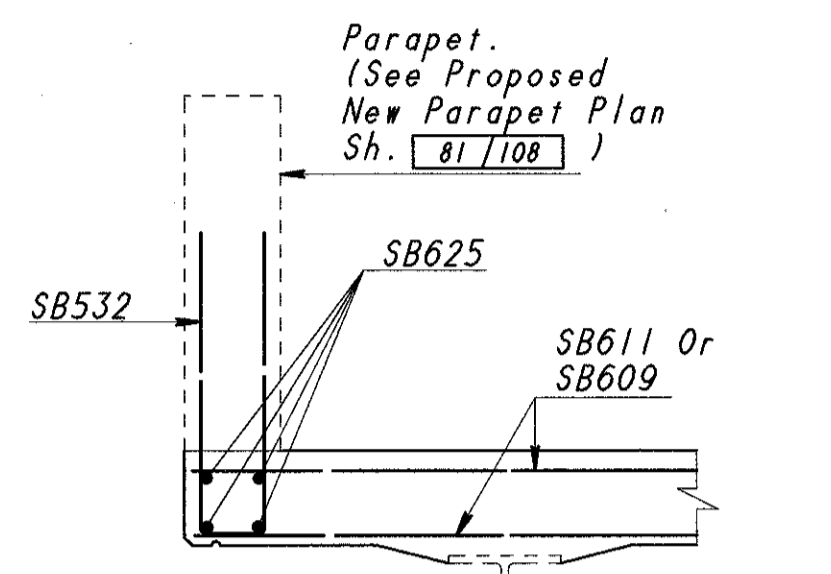
PLAN



SECTION B2-B2



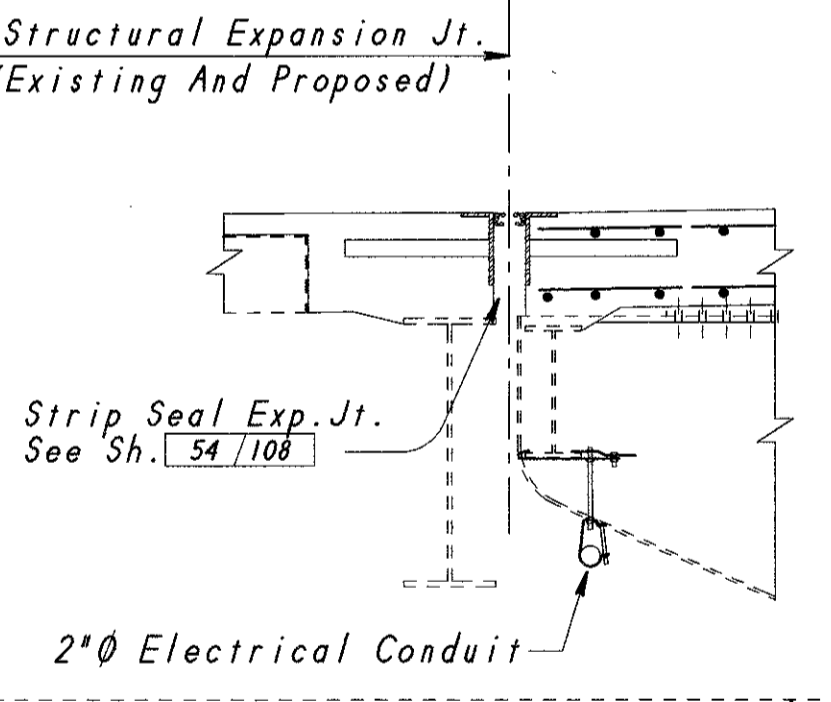
DETAIL M-M



SECTION MI-MI

NOTES

1. - Work This Sheet With General Plan Sh. 4/108
2. - See REINFORCING STEEL BAR LIST, Sh. 89/108
3. - See MISCELLANEOUS DETAILS, Sh. 12/108
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108
5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 77/108
6. - See Additional Details Sh. 60/108



Structural Expansion Jt. (Existing And Proposed)

Strip Seal Exp. Jt. See Sh. 54/108

2" Electrical Conduit

Existing Ramp A Concrete Deck To Be Partially Removed And Replaced As Part Of Stage I Construction. See Sh. 74/108

LEGEND
Existing Structure
Proposed Work

Pflum, Klausmeyer & Gehrm Consultants		61/108
SPAN 2-3 DECK SLAB UNIT B		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

NOTE - Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

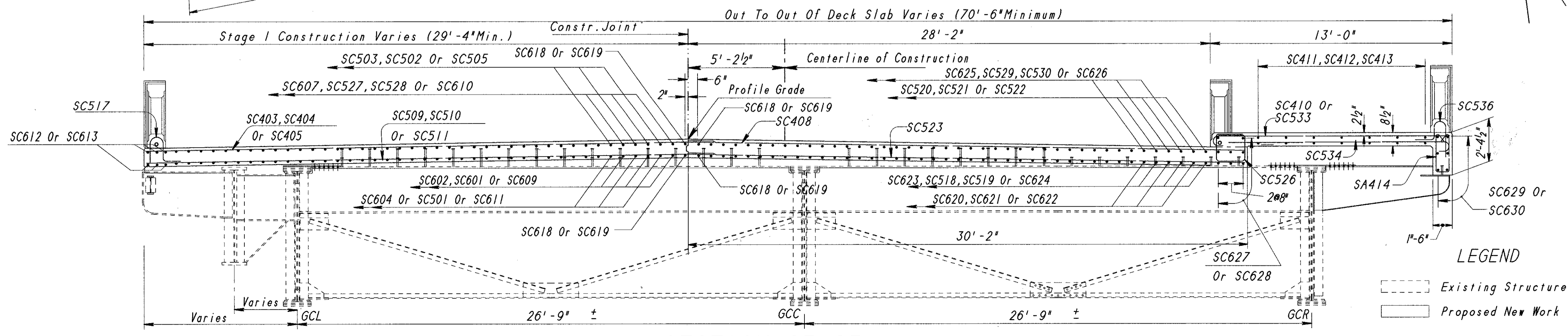
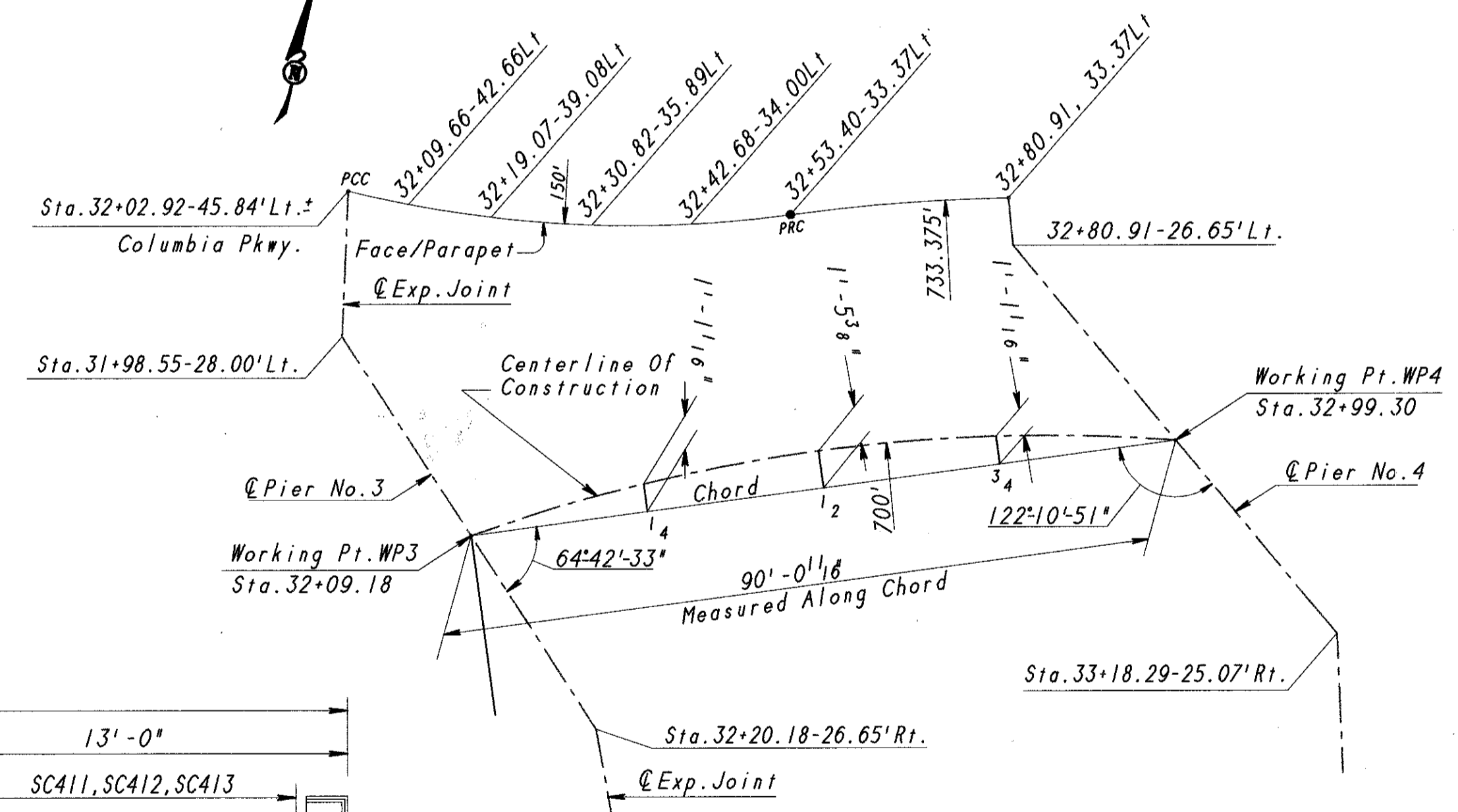
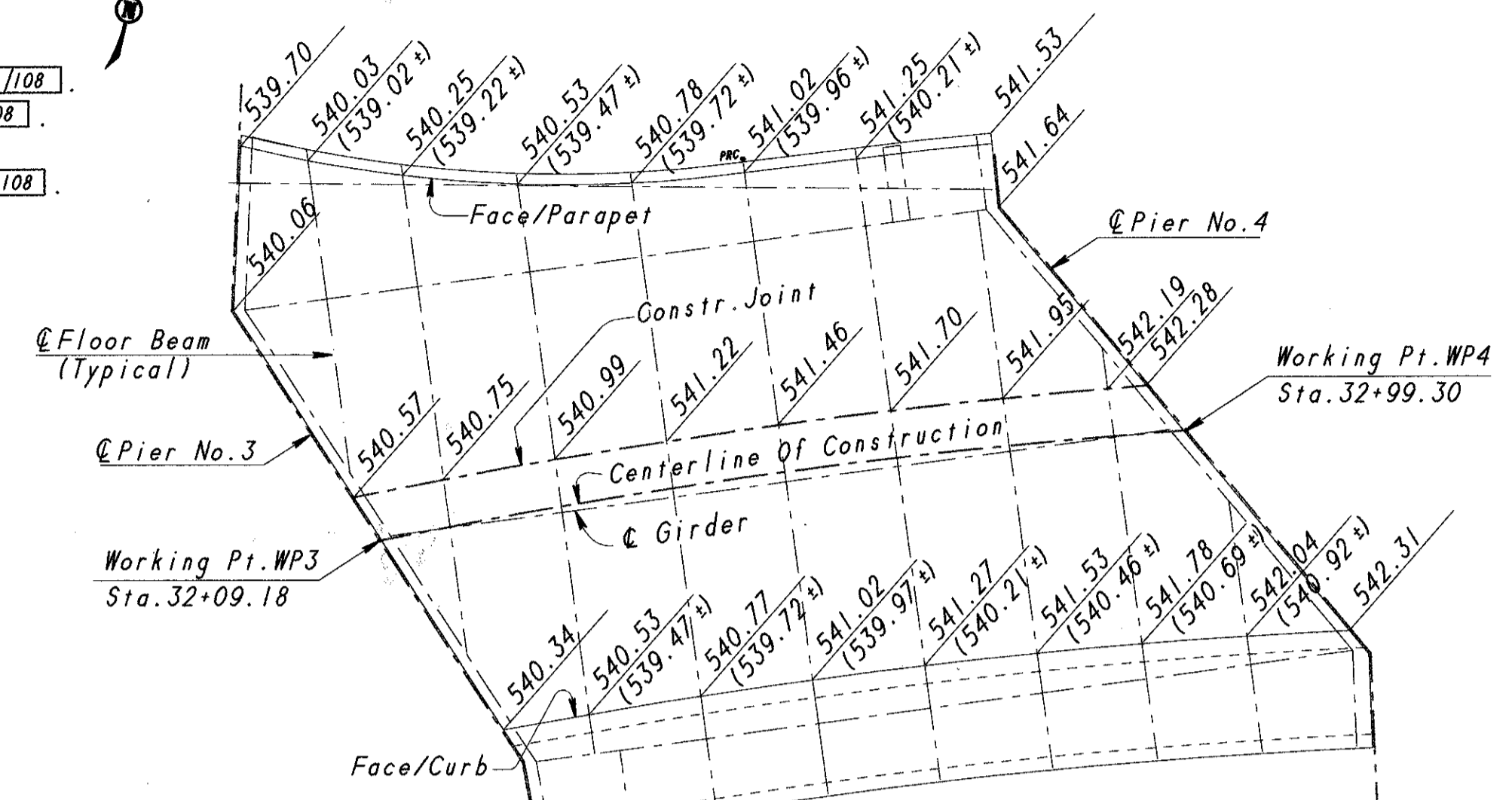
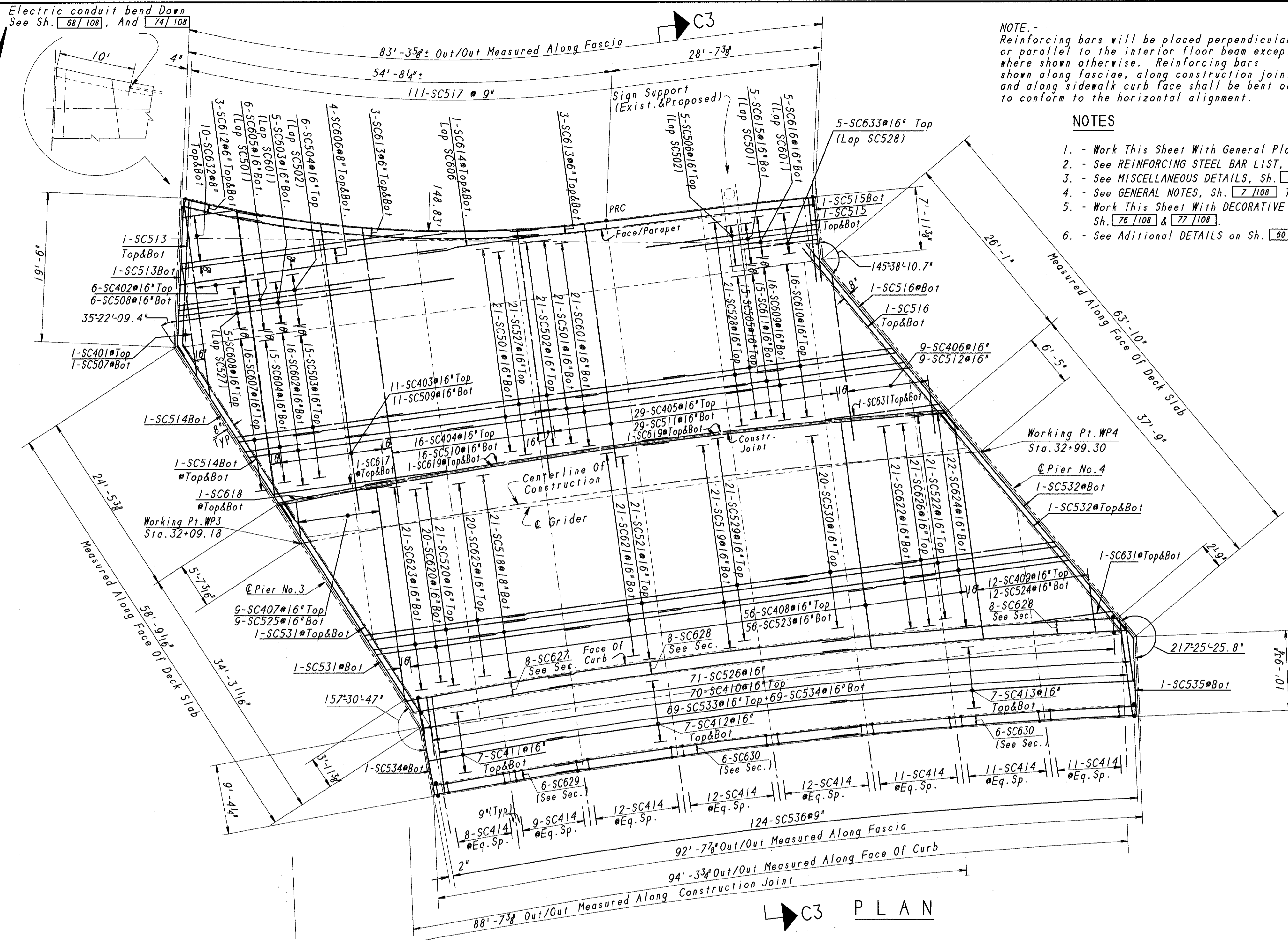
- NOTES
1. - Work This Sheet With General Plan Sh. 4/108
 2. - See REINFORCING STEEL BAR LIST, Sh. 90/108
 3. - See MISCELLANEOUS DETAILS, Sh. 12/108
 4. - See GENERAL NOTES, Sh. 2/108 Thru 11/108
 5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 77/108
 6. - See Additional DETAILS on Sh. 60/108

BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND
123.45..... Denotes Top Of Slab Elevation.
(123.45±)..... Denotes Top Of Existing Steel Elevation.



LEGEND
Existing Structure
Proposed New Work

Pfium, Klausmeier & Gehrm
Consultants
62/108
OHIO

SPAN 3-4
DECK SLAB UNIT C

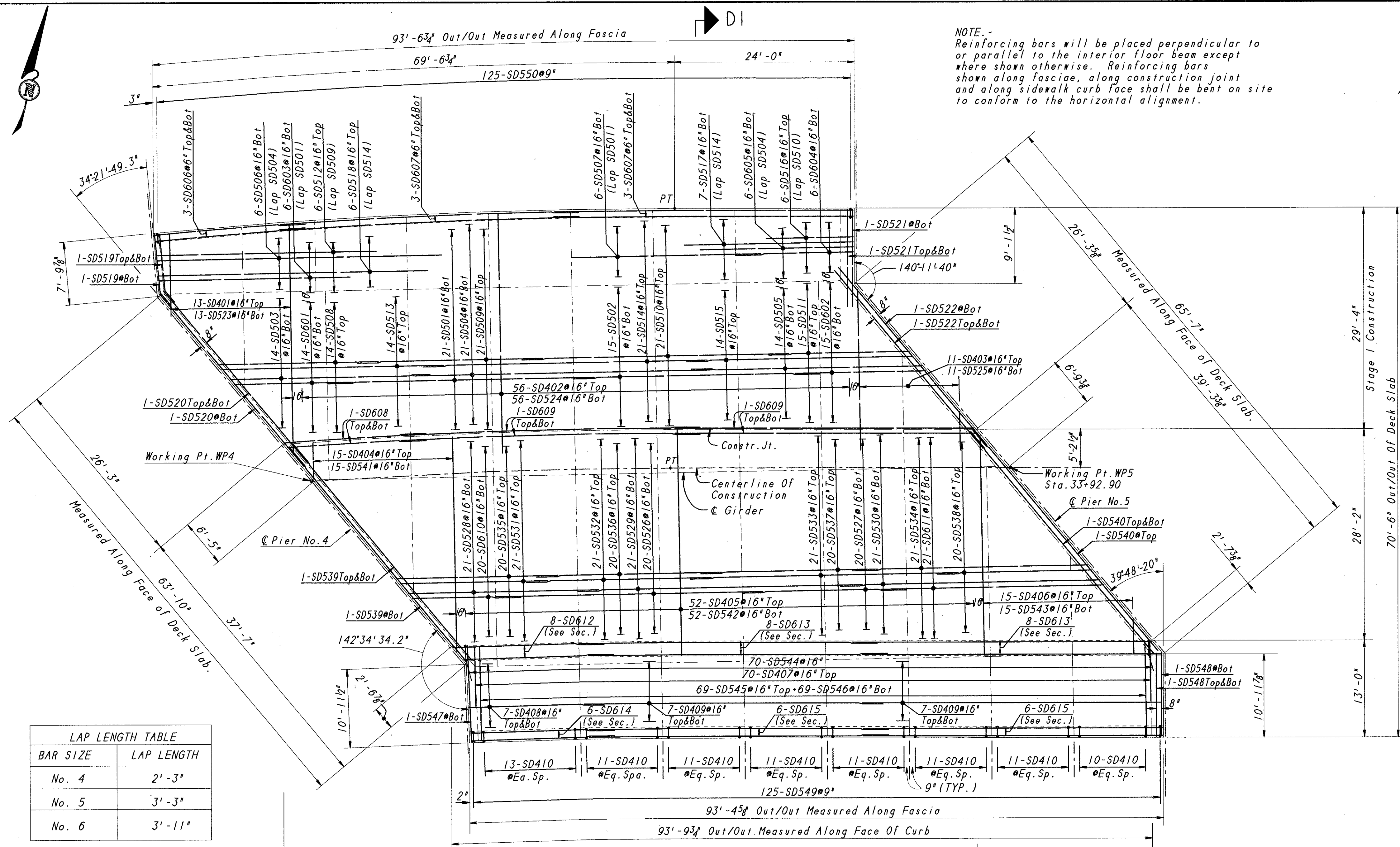
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
Sta. 30+55.40
Sta. 47+16.19

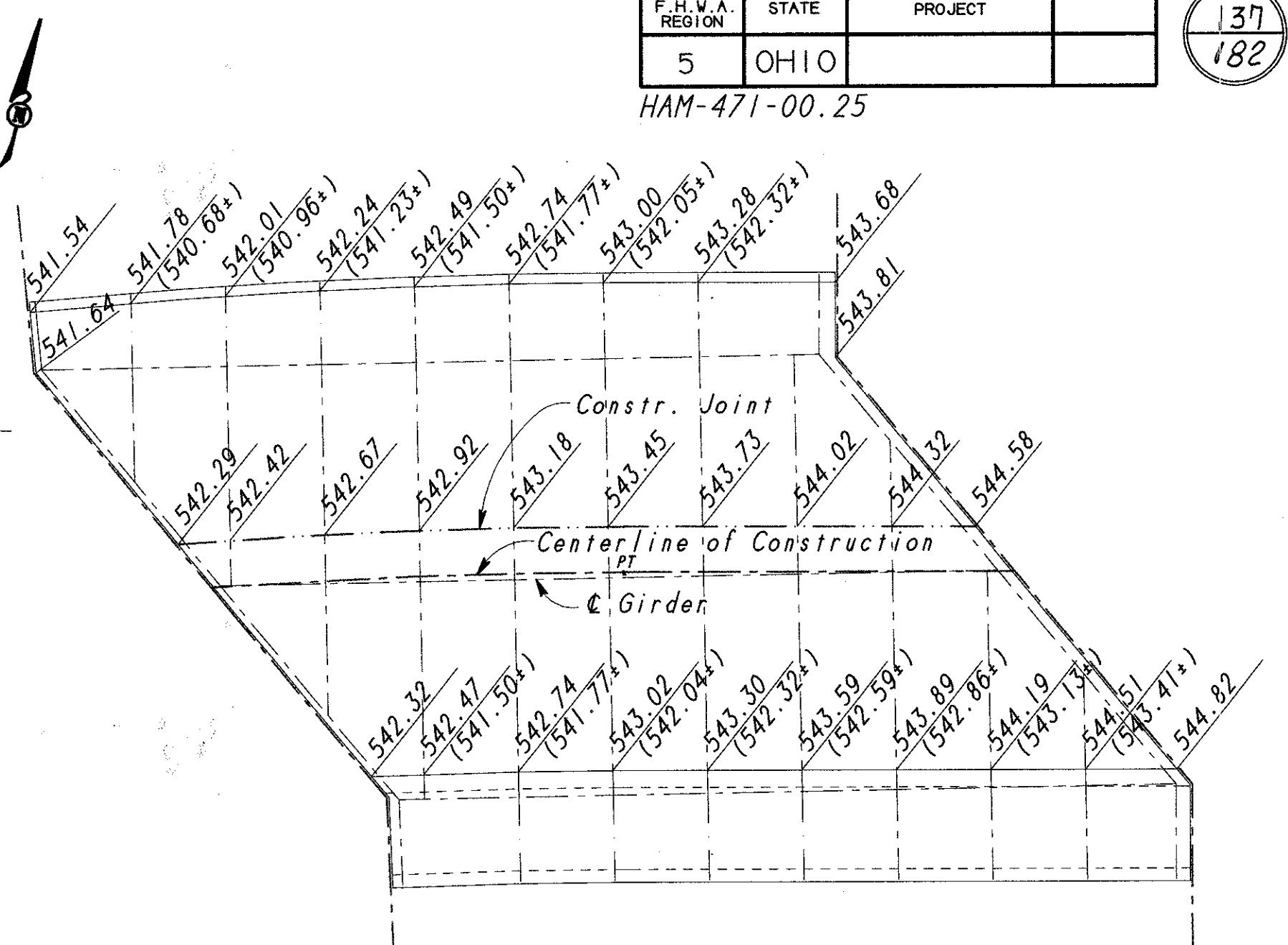
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED		WDD	IEH	April 96	

SECTION C3-C3

NOTE.-
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

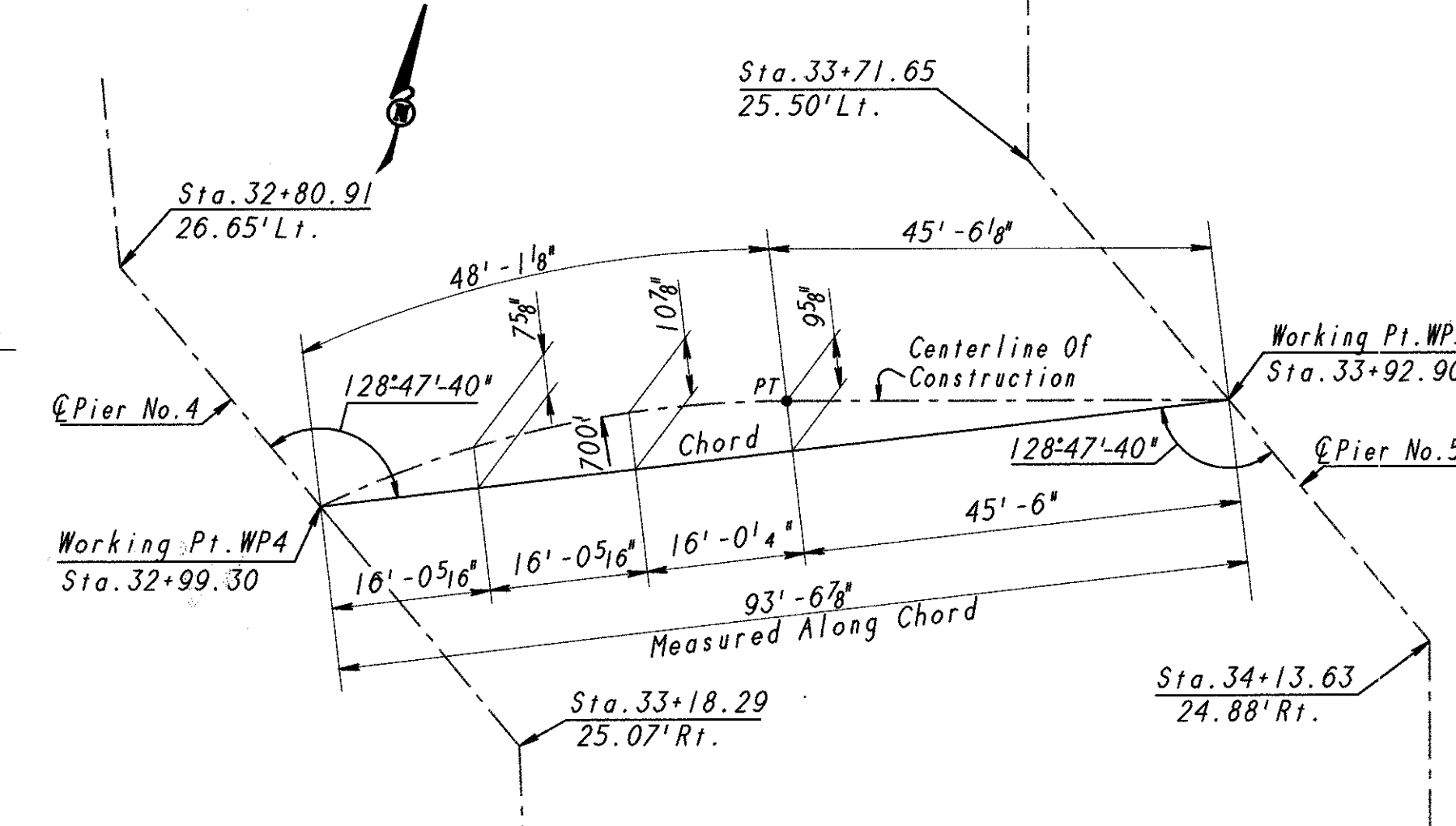


BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

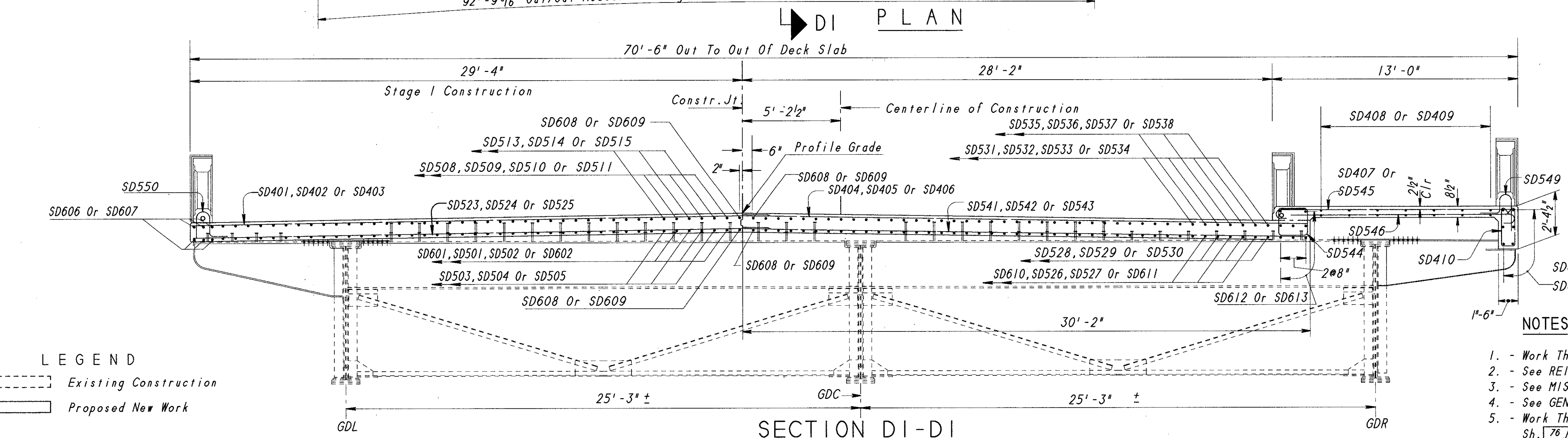


SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)
LEGEND
123.45.....Denotes Top Of Slab Elevation.
123.45±.....Denotes Top Of Existing Steel Elevation.



DECK SPAN SCHEMATIC LAYOUT PLAN



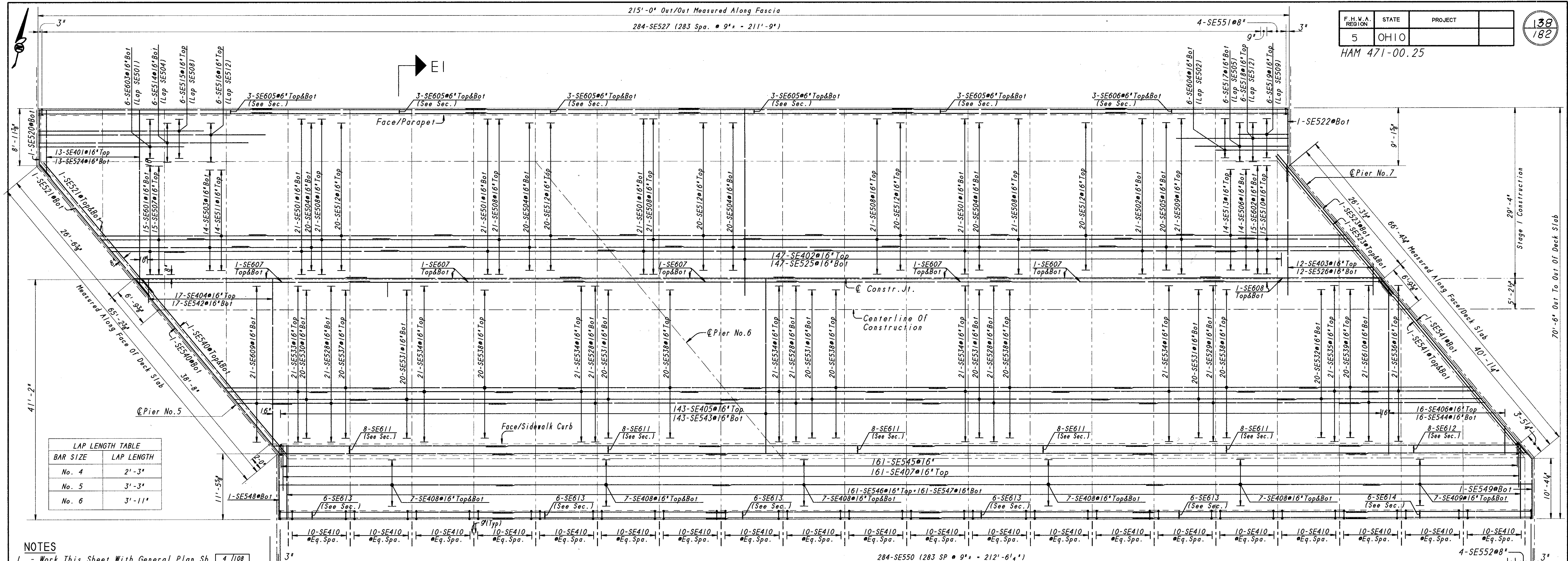
SECTION DI-DI

LEGEND
Existing Construction
Proposed New Work

- NOTES
1. - Work This Sheet With General Plan Sh. 4/108.
 2. - See REINFORCING STEEL BAR LIST, Sh. 9/1108.
 3. - See MISCELLANEOUS DETAILS, Sh. 12/1108.
 4. - See GENERAL NOTES, Sh. 7/1108 Thru 11/1108.
 5. - Work This Sheet With DECORATIVE RAILING Sh. 76/1108 & 77/1108.

Plum, Klausmeier & Gehrm Consultants		63/108
SPAN 4-5 DECK SLAB UNIT D		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

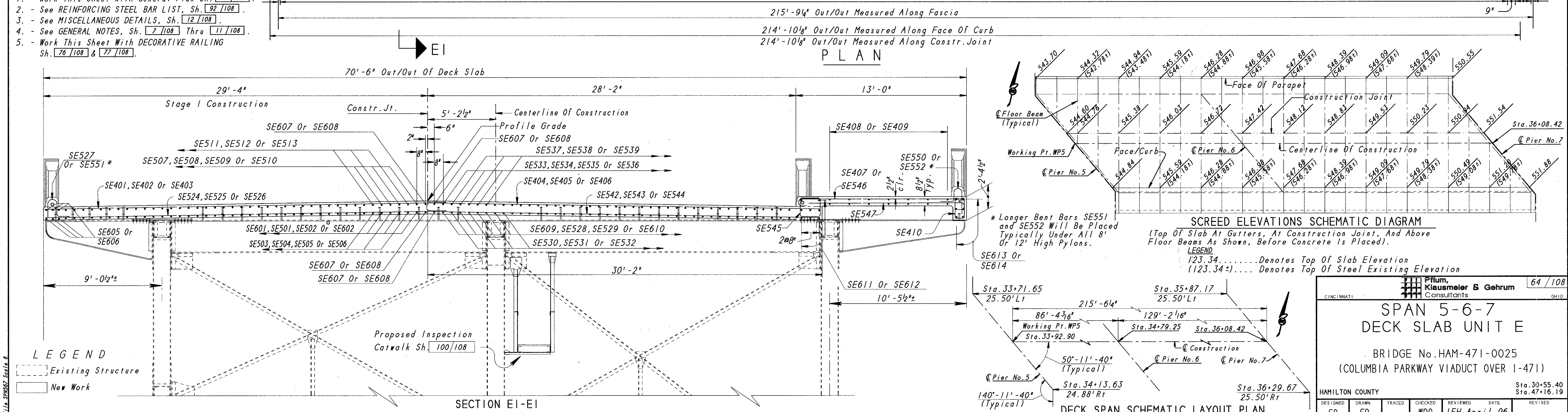
SPN-SPM5 Scale 8



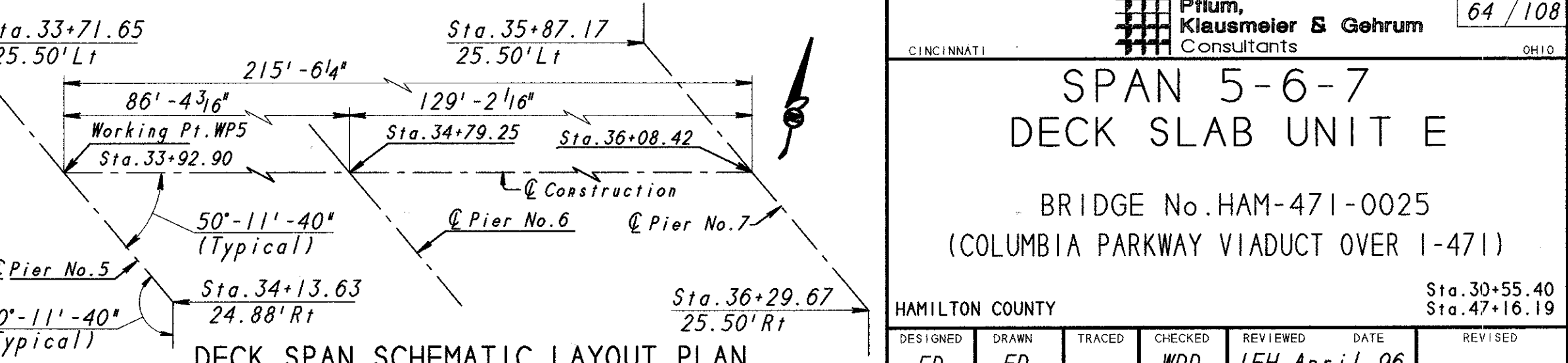
LAP LENGTH TABLE

BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

- NOTES
1. - Work This Sheet With General Plan Sh. 4/108.
 2. - See REINFORCING STEEL BAR LIST, Sh. 92/108.
 3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
 4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
 5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 77/108.



SCREED ELEVATIONS SCHEMATIC DIAGRAM
(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed).
LEGEND
123.34..... Denotes Top Of Slab Elevation
(123.34±).... Denotes Top Of Steel Existing Elevation



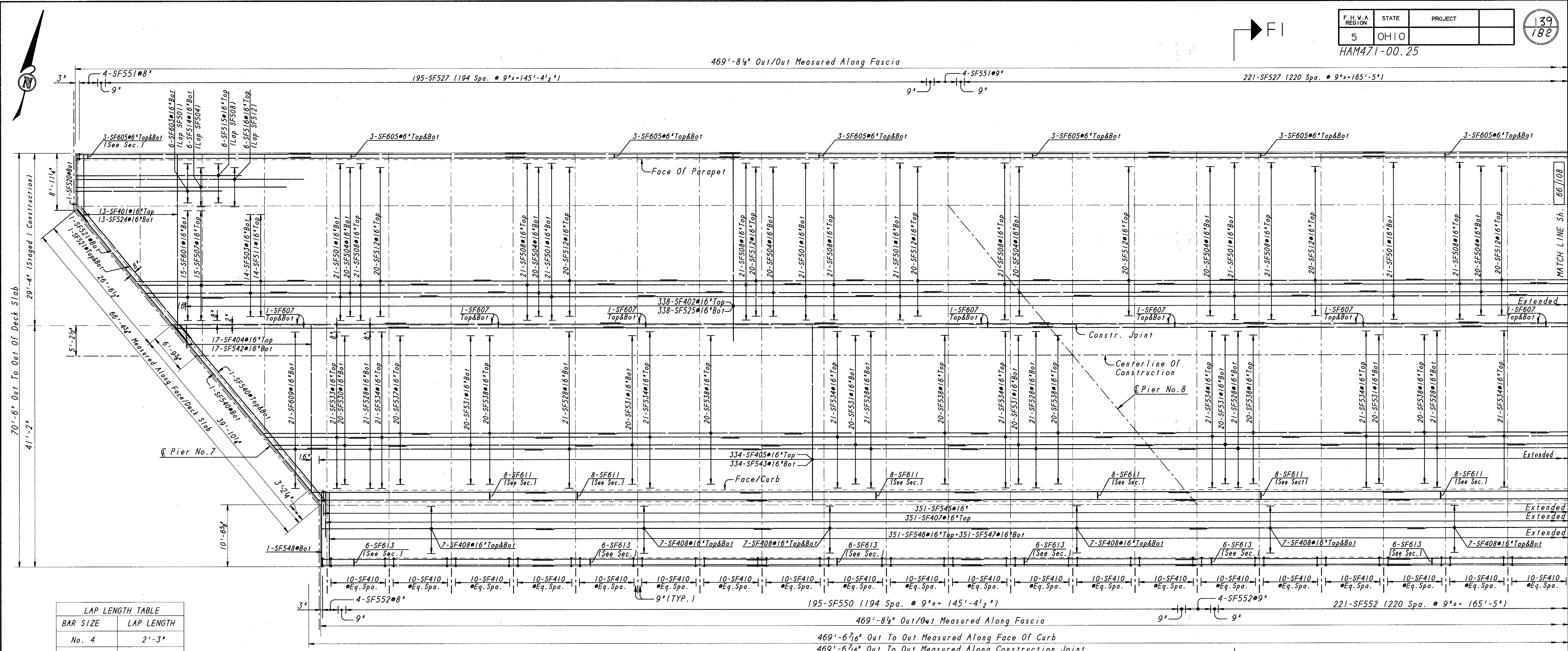
64/108
Klauser & Gehrm
Consultants
OHIO

SPAN 5-6-7
DECK SLAB UNIT E

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED	WDD	IEH	April 96		

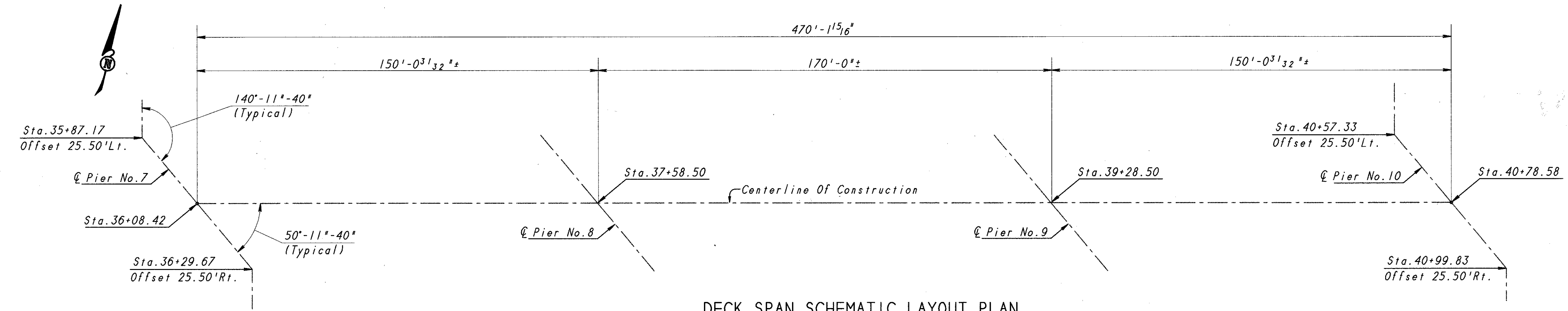


BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

PLAN (PART 1/2)

NOTES

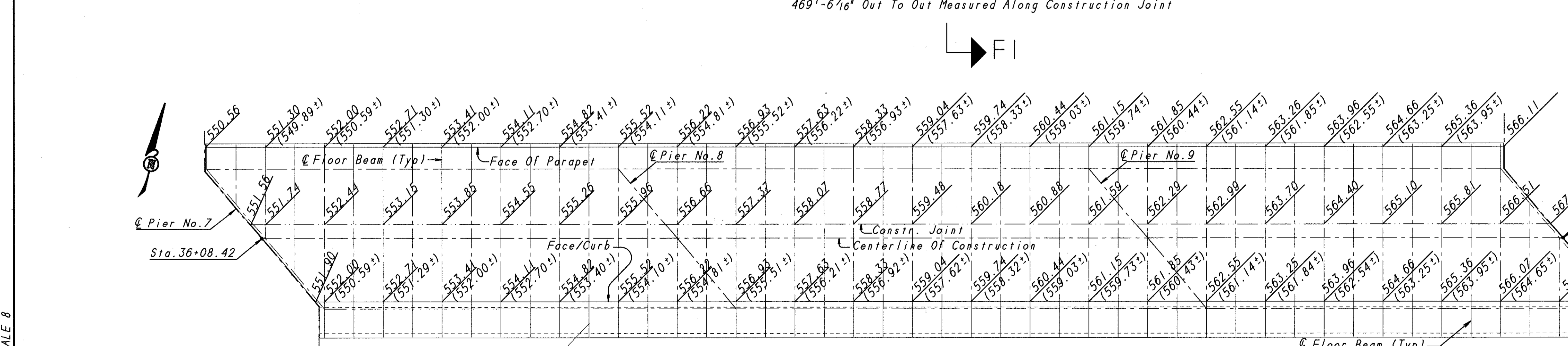
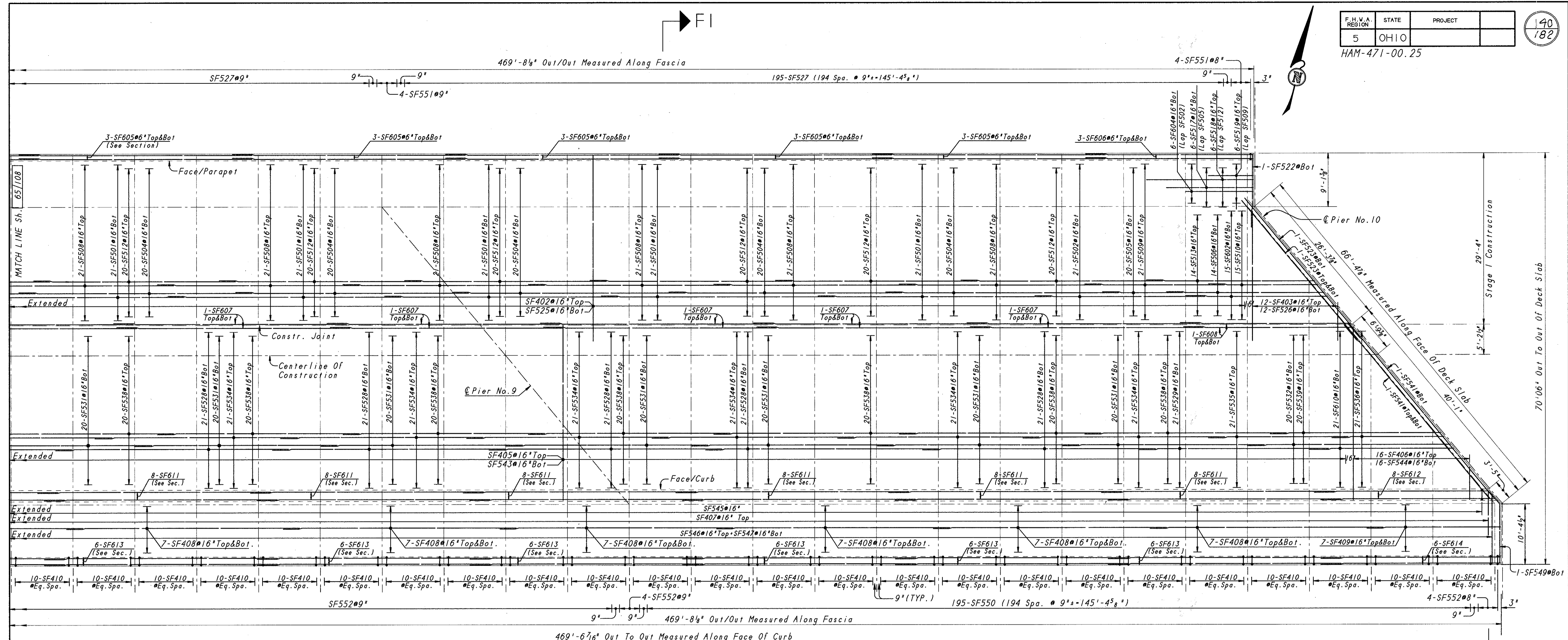
1. - Work This Sheet With General Plan Sh. 4/108 & Sh. 5/108.
2. - Work This Sheet With Sh. 67/108.
3. - See REINFORCING STEEL BAR LIST, Sh. 93/108.
4. - See MISCELLANEOUS DETAILS, Sh. 12/108.
5. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
6. - Work This Sheet With DECORATIVE RAILING, Sh. 76/108 and Sh. 28/108.
7. - For Section F1-F1 see similar section E1-E1 on Sh. 64/108 DECK SLAB SPAN 5-6-7



DECK SPAN SCHEMATIC LAYOUT PLAN

Plum, Klausmeyer & Gehrm Consultants		65/108
SPAN 7-8-9-10 DECK SLAB UNIT F 1/2 BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	CHECKED WDD
REVIEWED IEH	DATE April 96	REVISIONS

1-3 DECK 15P78910A SCALE 8



SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND
123.45.....Denotes Top Of Slab Elevation.
(123.45±).....Denotes Top Of Existing Steel Elevation.

BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

NOTES

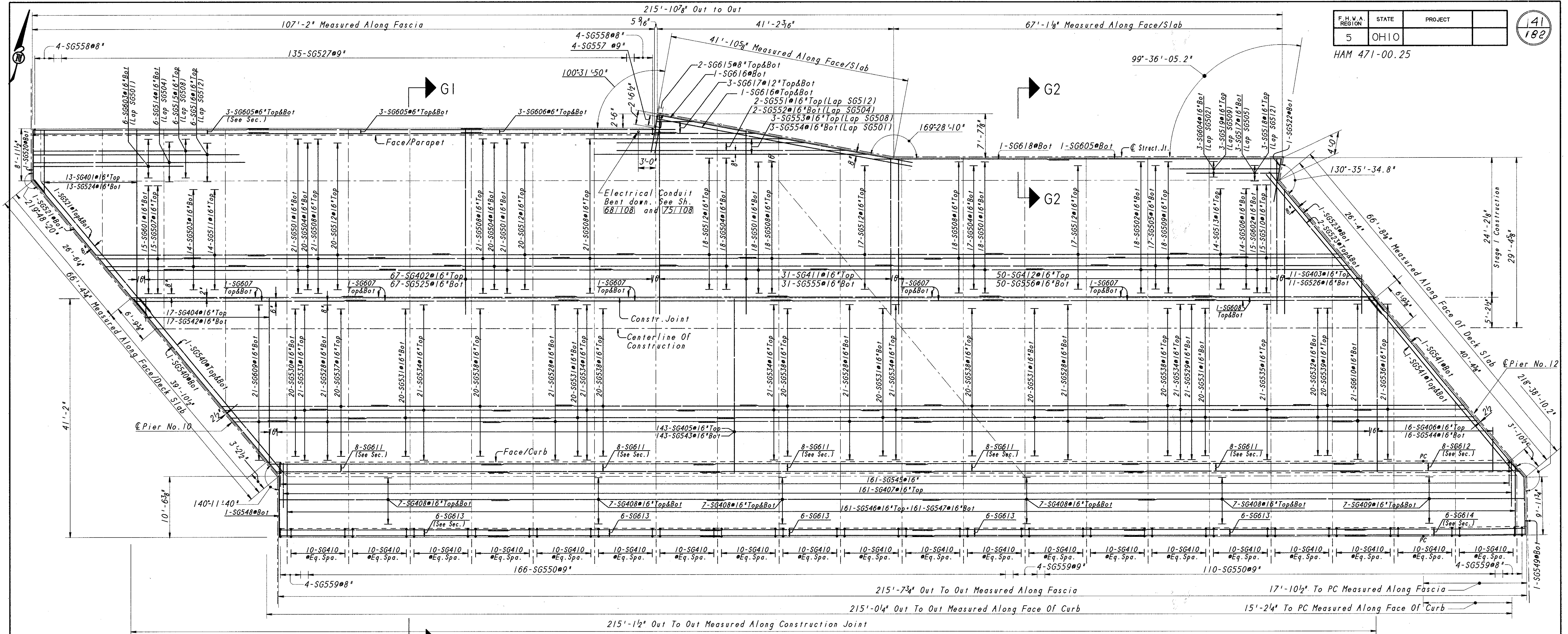
1. - Work This Sheet With General Plan Sh. 4/108 & 5/108.
2. - Work This Sheet With Sh. 65/108.
3. - See REINFORCING STEEL BAR LIST, Sh. 93/108.
4. - See MISCELLANEOUS DETAILS, Sh. 12/108.
5. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
6. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 78/108.
7. - For Section FI-FI see similar section EI-EI on Sh. 64/108 DECK SLAB SPAN 5-6-7

Plum, Klausmeier & Gehrm
Consultants
66/108

SPAN 7-8-9-10
DECK SLAB UNIT F
2/2
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY	DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISION
	ED	ED		WDD	1EH	April 96	

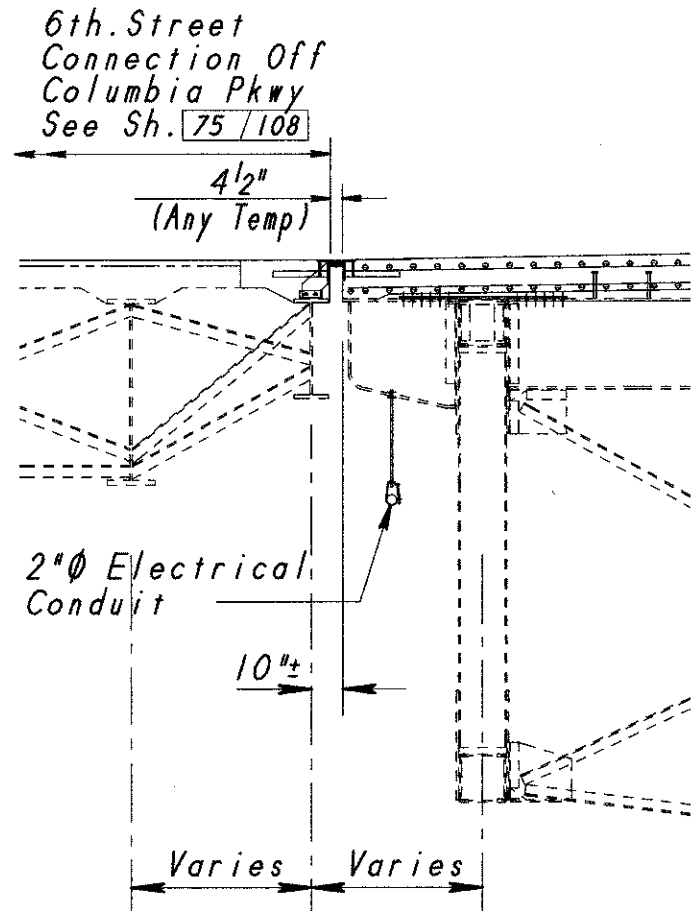
SCALE 8/16



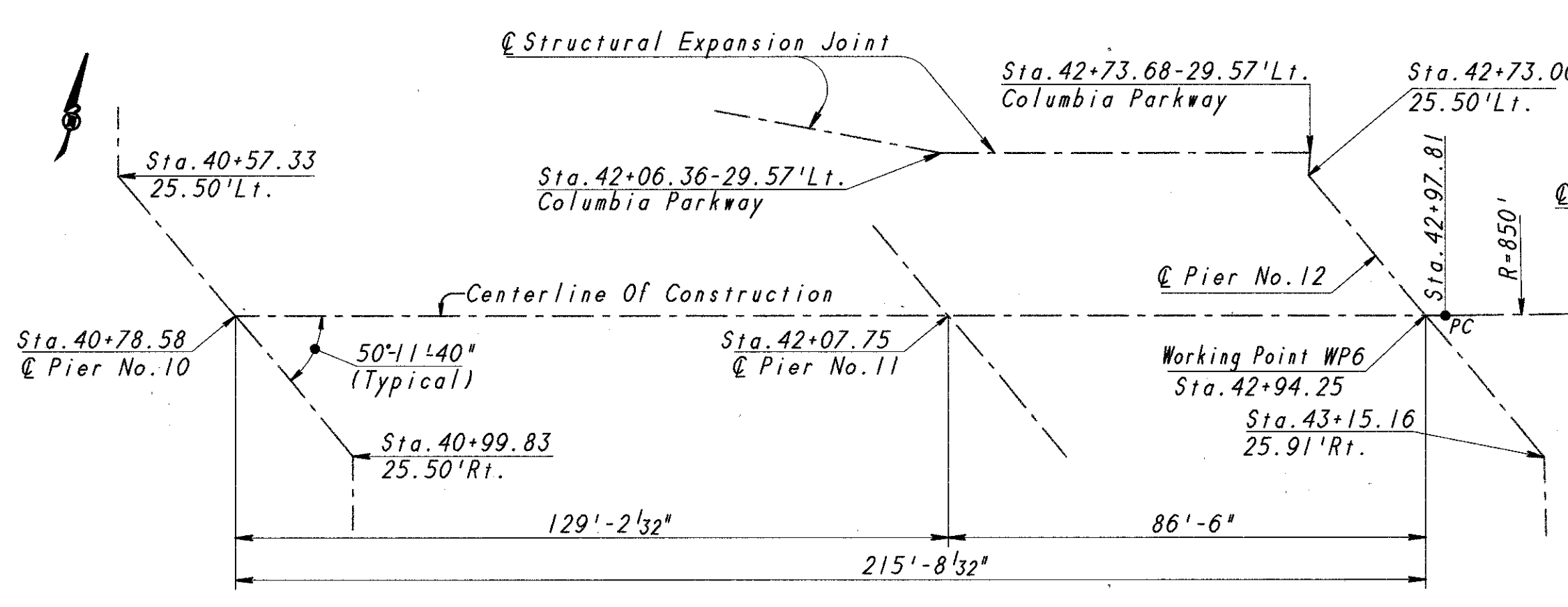
PLAN

NOTES

1. - Work This Sheet With General Plan Sh. 5/108.
2. - See REINFORCING STEEL BAR LIST, Sh. 94/108.
3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 79/108.
6. - For Section G1-G1 See Similar Section E1-E1 On Sh. 64/108 DECK SLAB SPAN 5-6-7



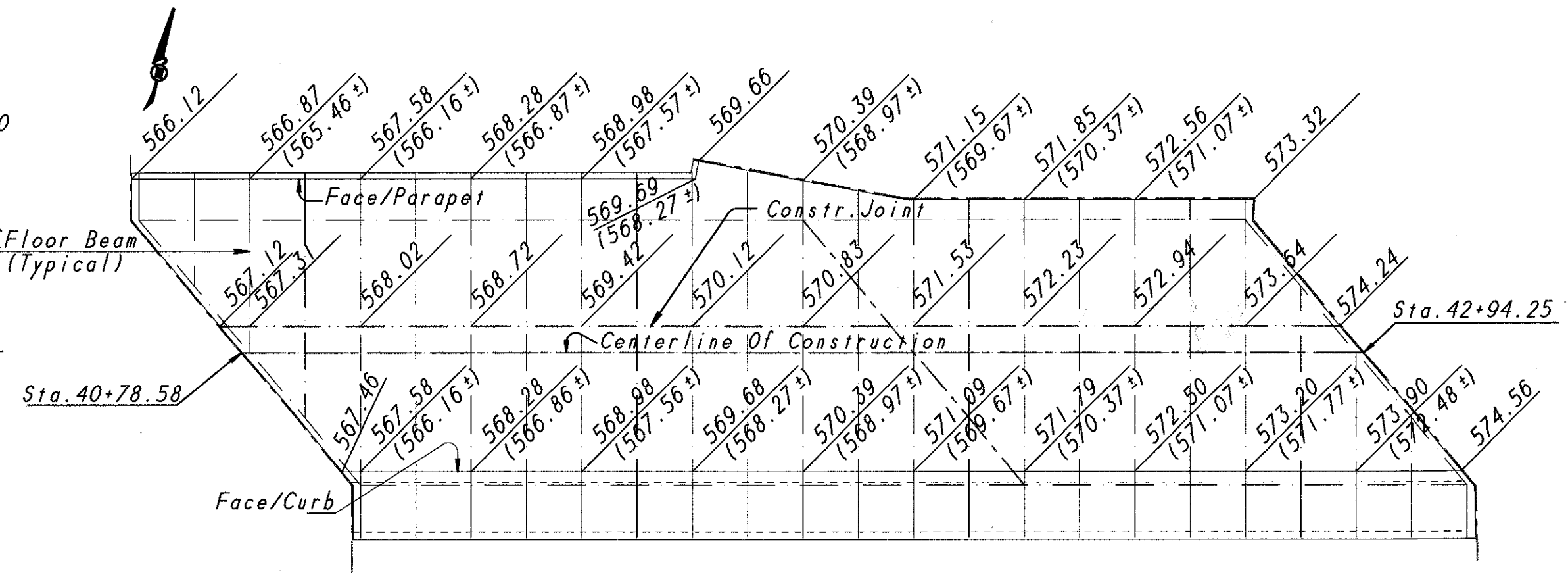
SECTION G2-G2



DECK SPAN SCHEMATIC LAYOUT PLAN

LEGEND

- Existing Structure
- New Work



SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND

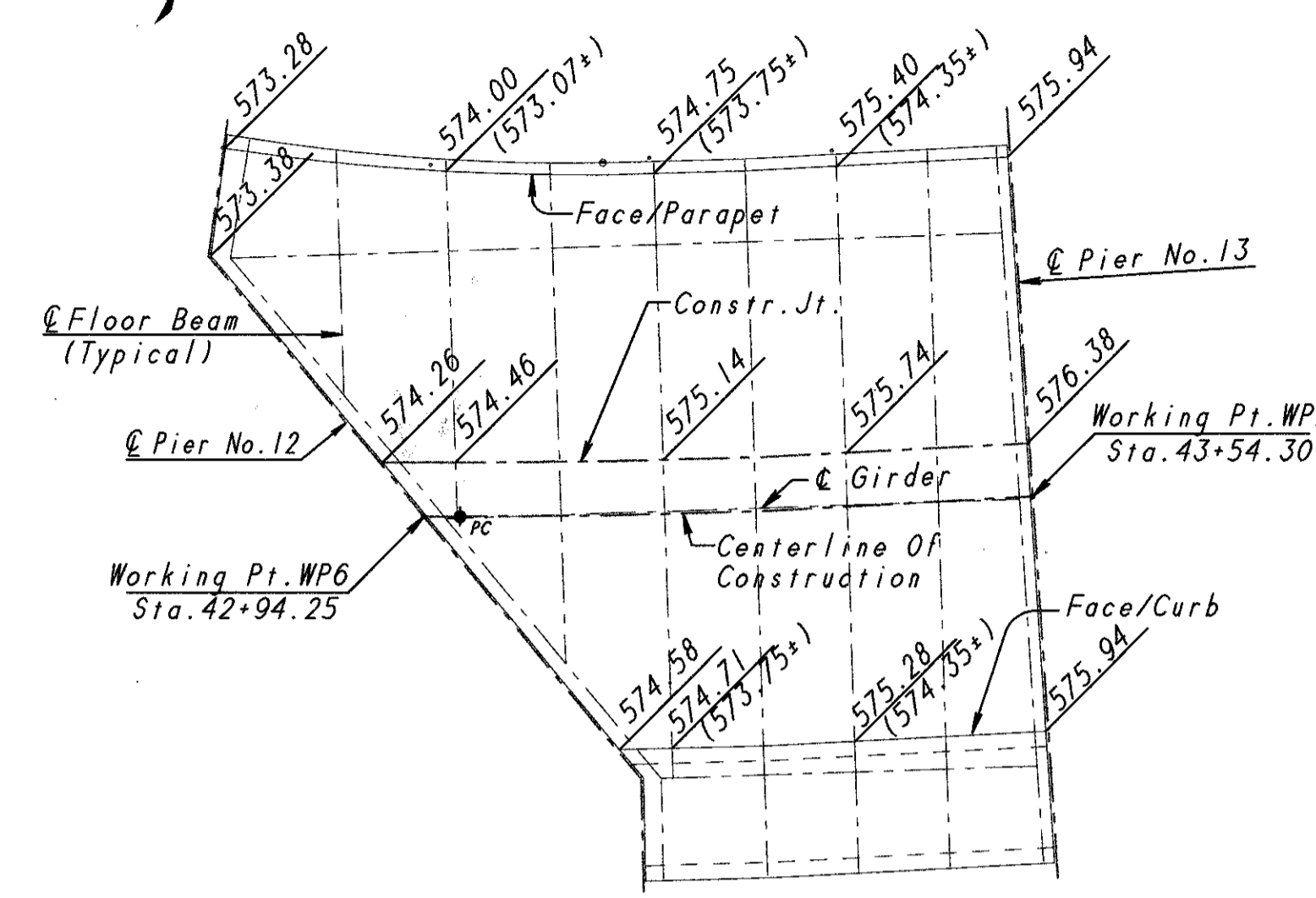
- 123.45.....Denotes Top Of Slab Elevation.
- (123.45).....Denotes Top Of Existing Steel Elevation

Plum, Klausmeier & Gehrm Consultants		67/108
SPAN 10-11-12 DECK SLAB UNIT G		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96
REVISED		

File SPl01112 Scale 8

NOTE - Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

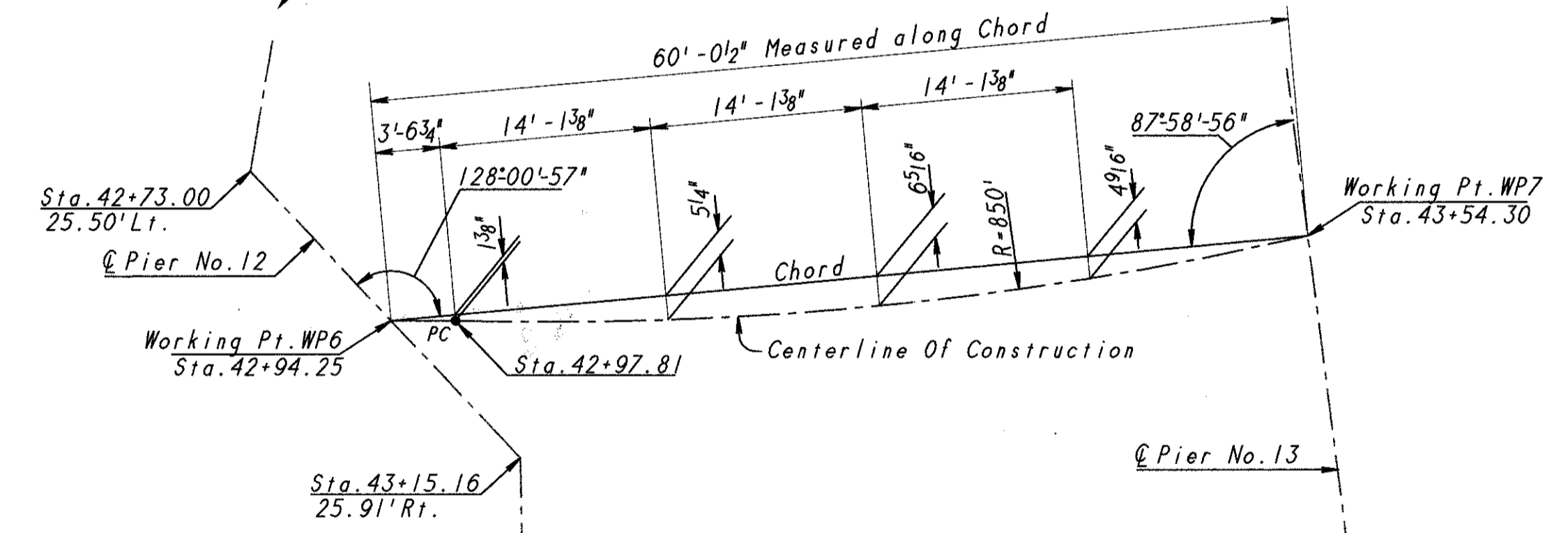
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"



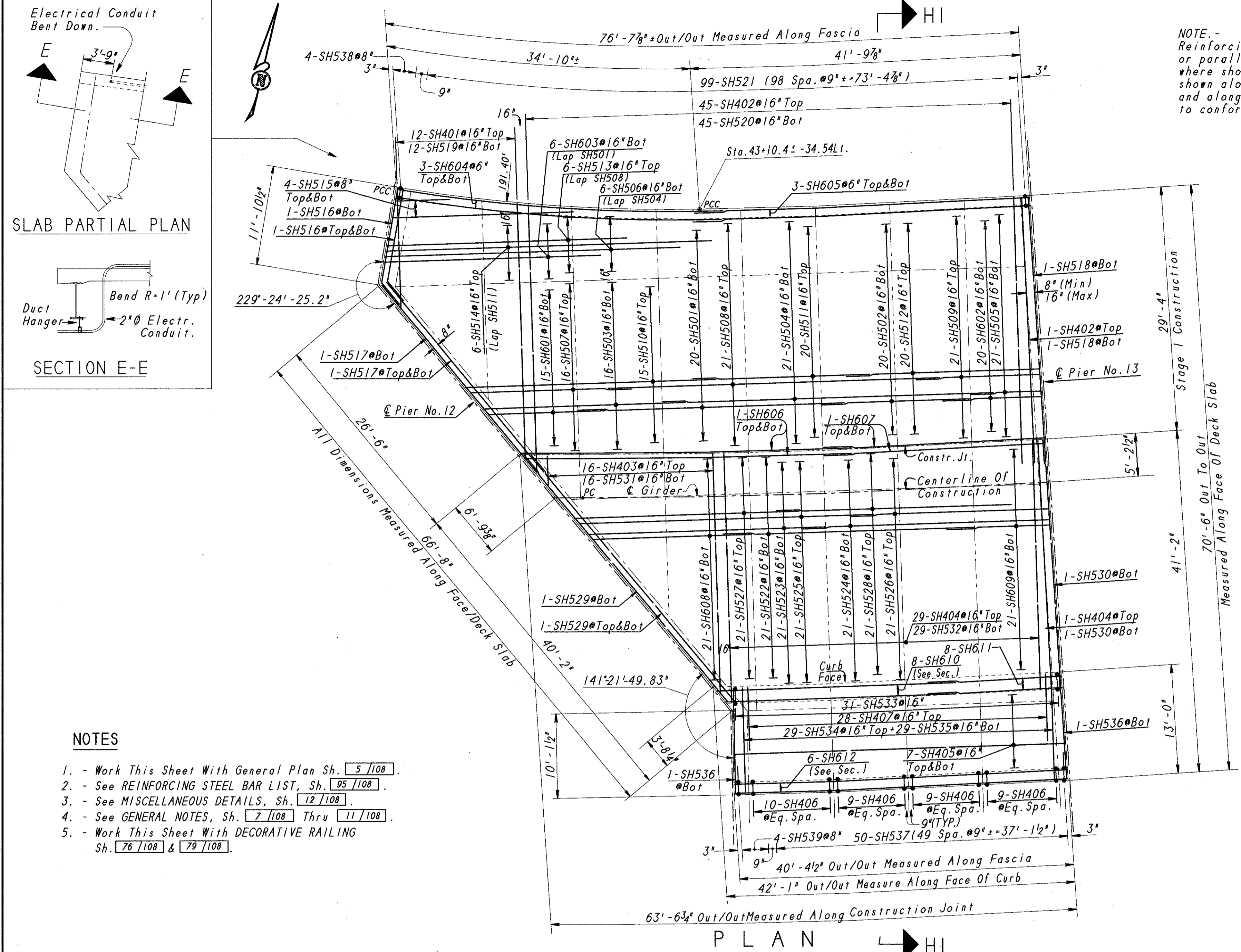
SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND
123.34..... Denotes Top Of Slab Elevation
(123.34).... Denotes Top Of Existing Steel Elevation



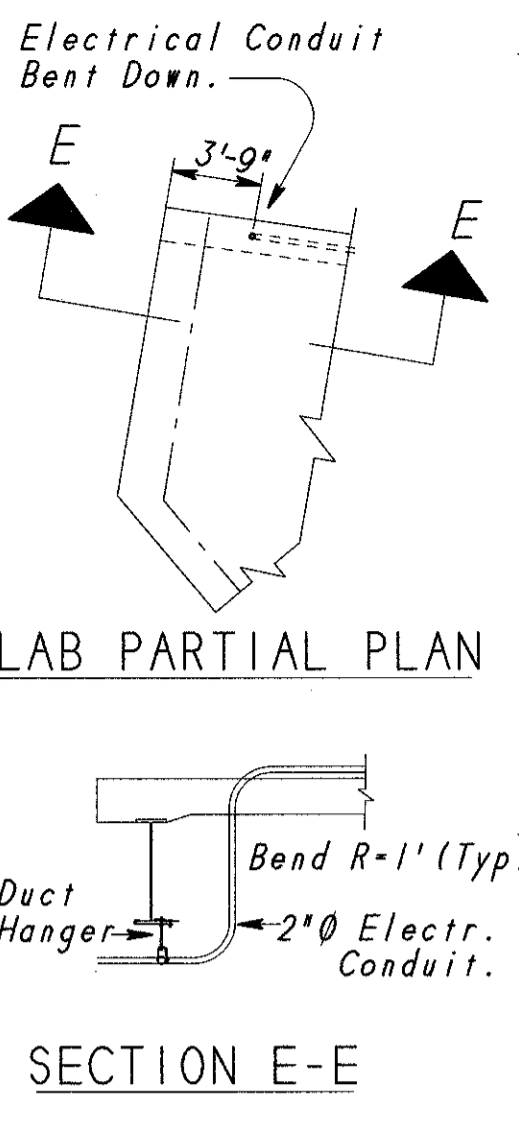
DECK SPAN SCHEMATIC LAYOUT PLAN



PLAN

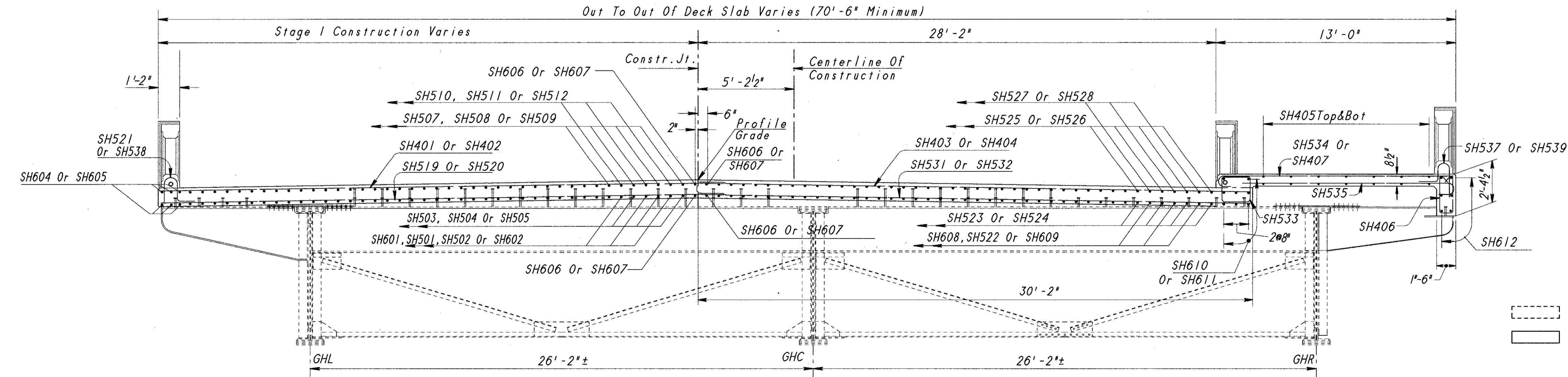
NOTES

1. - Work This Sheet With General Plan Sh. 5/108
2. - See REINFORCING STEEL BAR LIST, Sh. 95/108
3. - See MISCELLANEOUS DETAILS, Sh. 12/108
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108
5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 79/108



SLAB PARTIAL PLAN

SECTION E-E



SECTION HI-HI

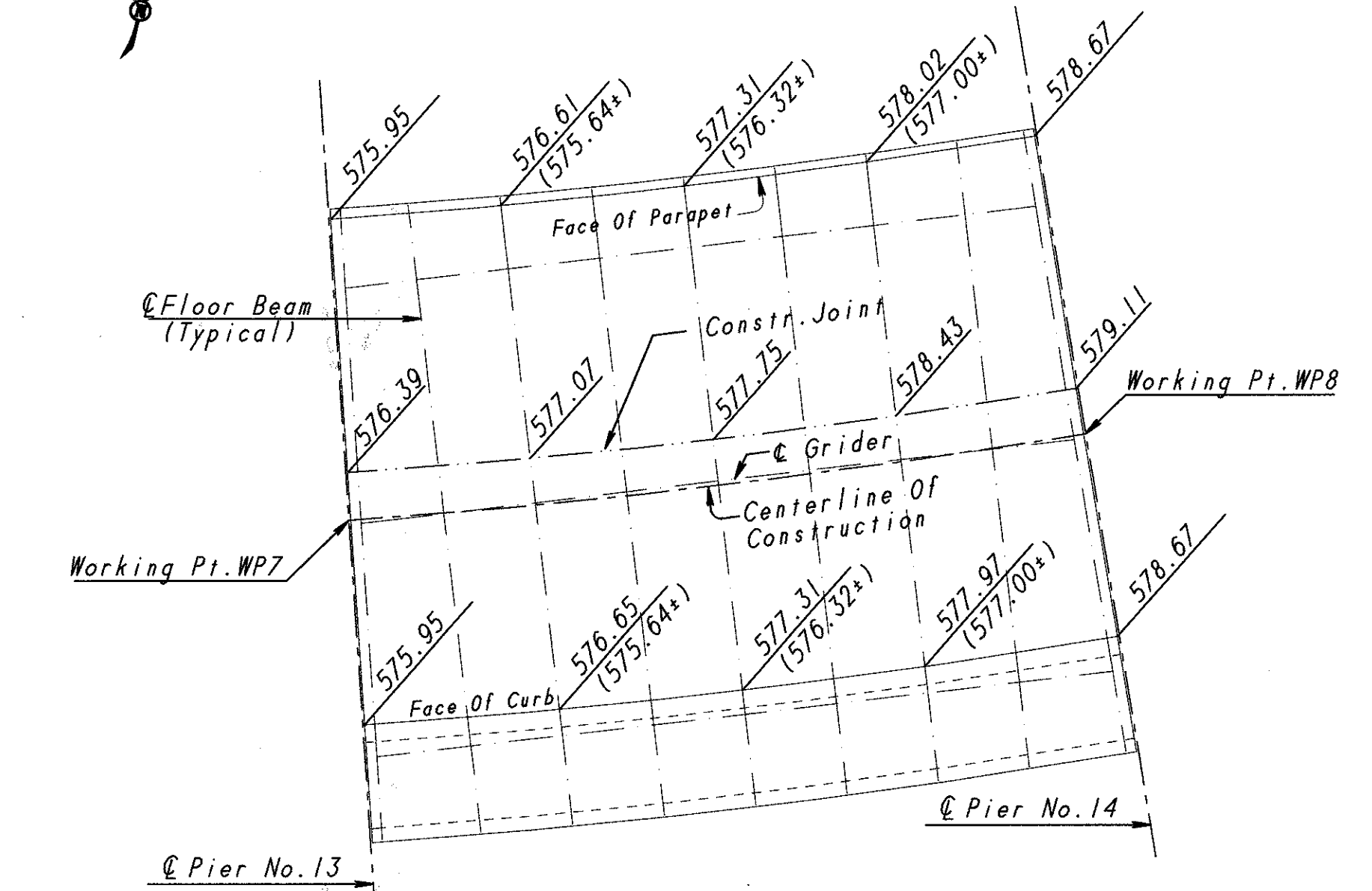
LEGEND
Existing Structure
New Work

Plum, Klausmeier & Gehrm Consultants		68 / 108
SPAN 12-13 DECK SLAB UNIT H		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

File name: SPN1213 Scale: 8

NOTE -
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

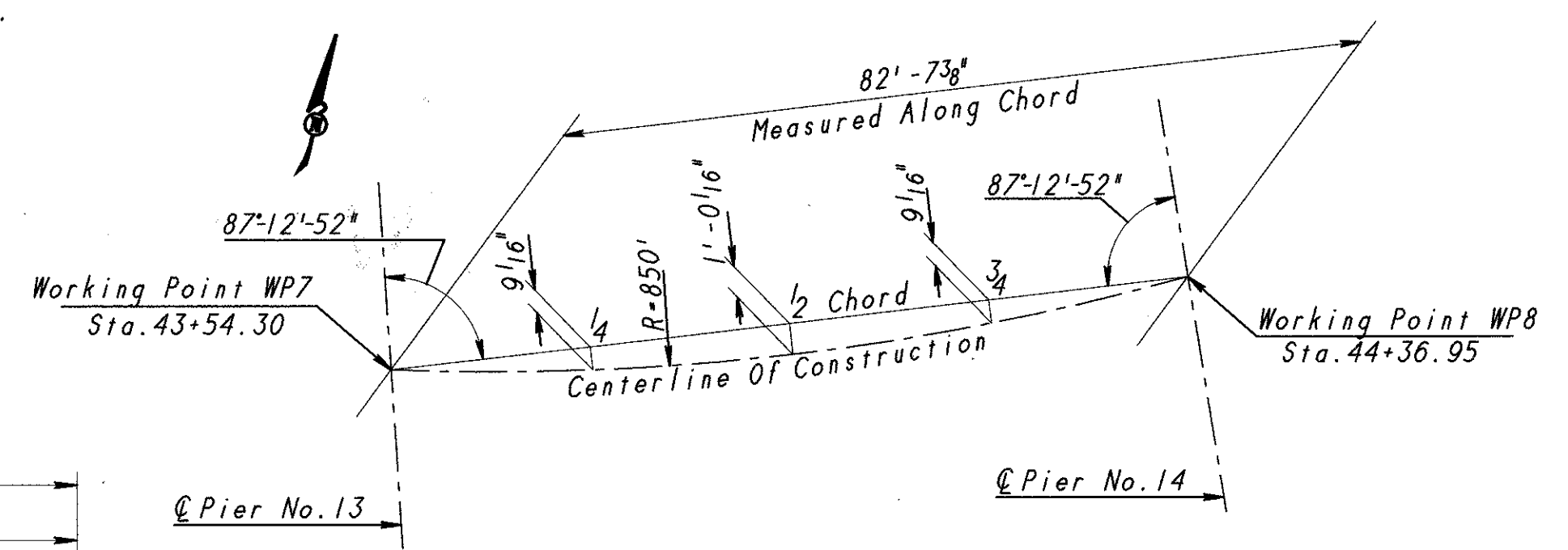
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"



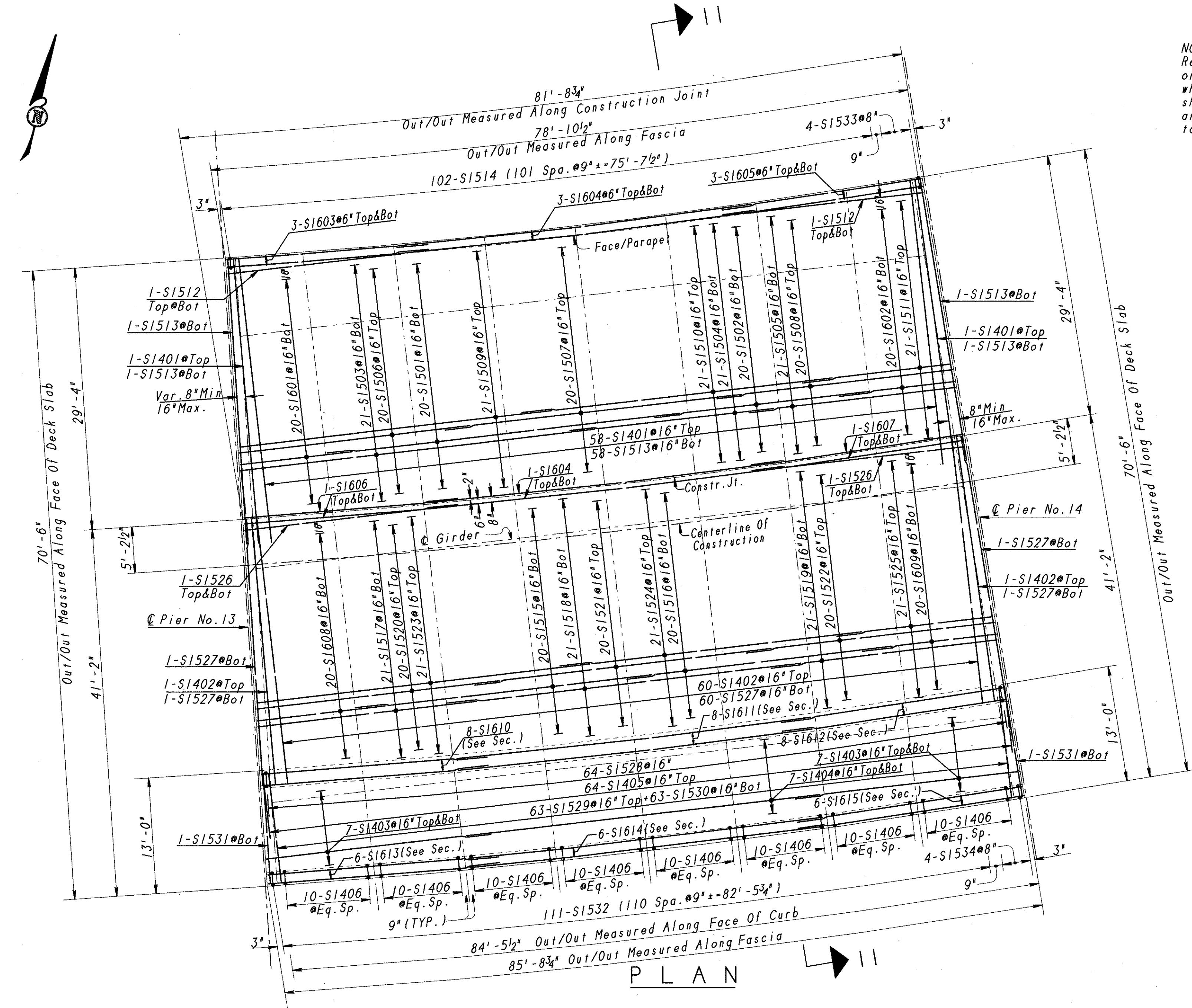
SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

LEGEND
123.45.....Denotes Top of Slab Elevation.
(123.45).....Denotes Top of Existing Steel Elevation.



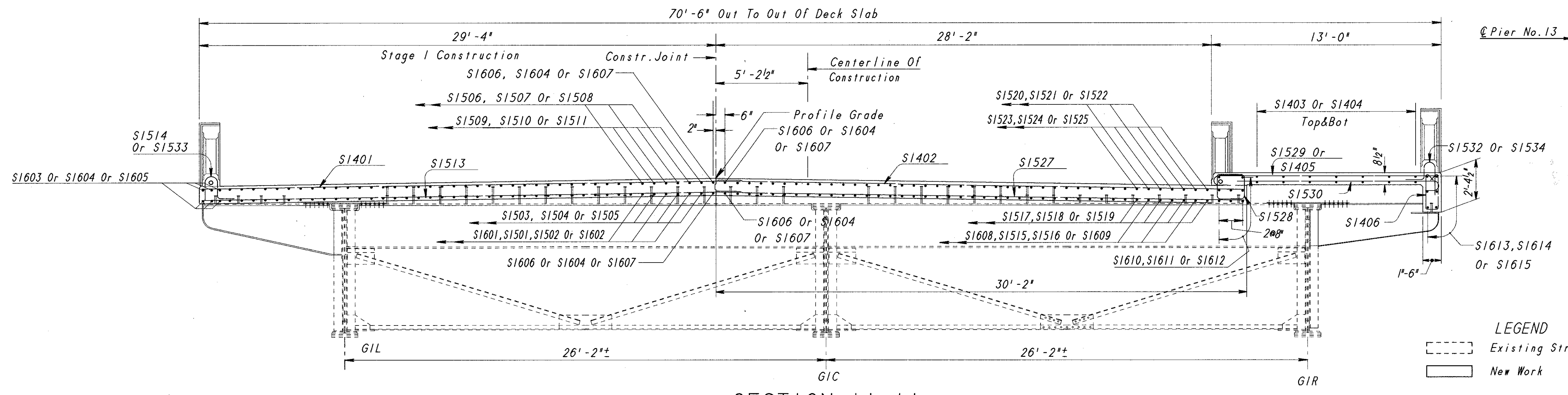
DECK SPAN SCHEMATIC LAYOUT PLAN



PLAN

NOTES

1. - Work This Sheet With General Plan Sh. 5/108 & 6/108.
2. - See REINFORCING STEEL BAR LIST, Sh. 96/108.
3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
5. - Work This Sheet With DECORATIVE RAILING Sh. 78/108 & 79/108.



SECTION II-II

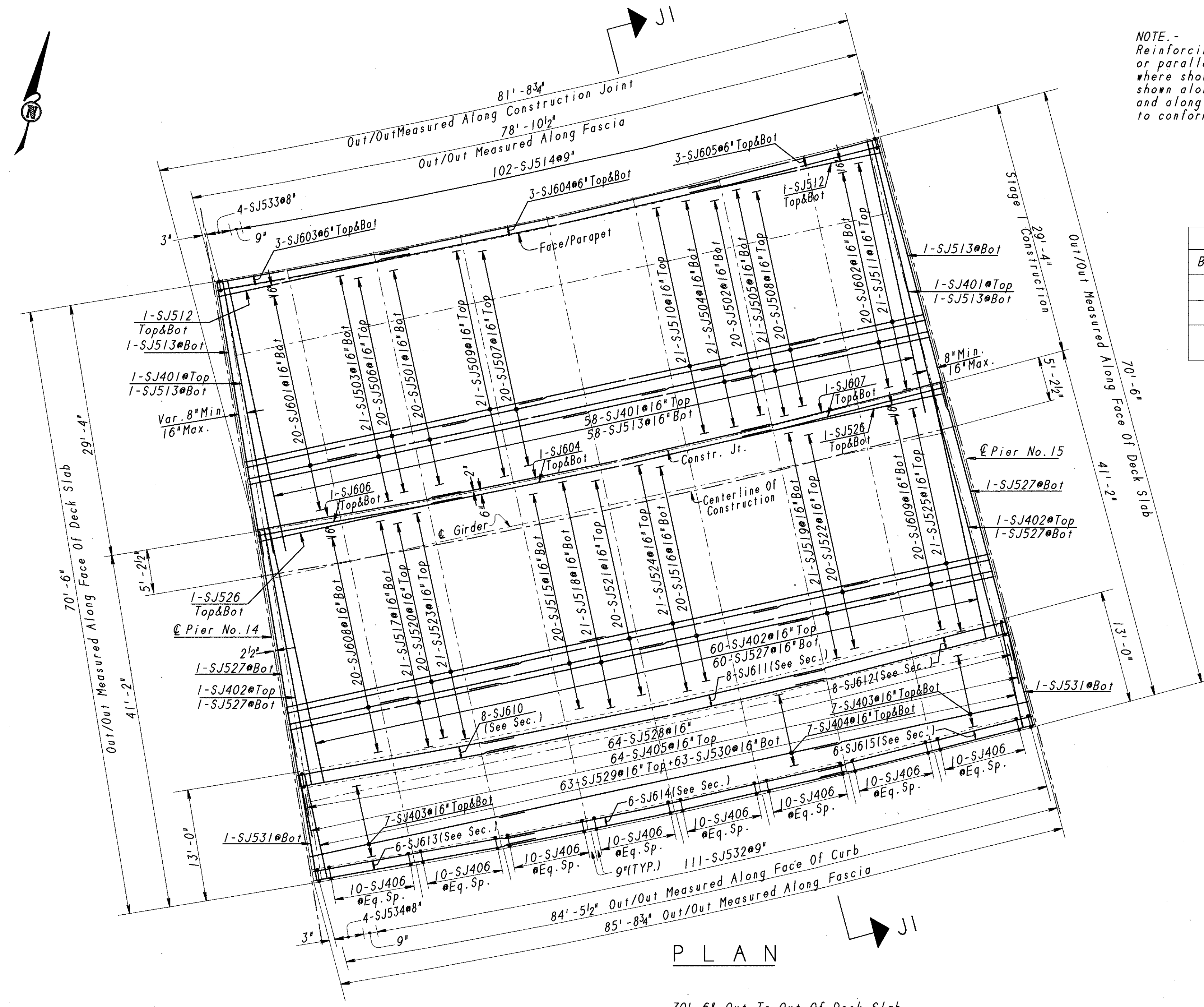
LEGEND
Existing Structure
New Work

Flum, Klausmeier & Gehrm Consultants		69/108
SPAN 13-14 DECK SLAB UNIT I		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96
		REVISED

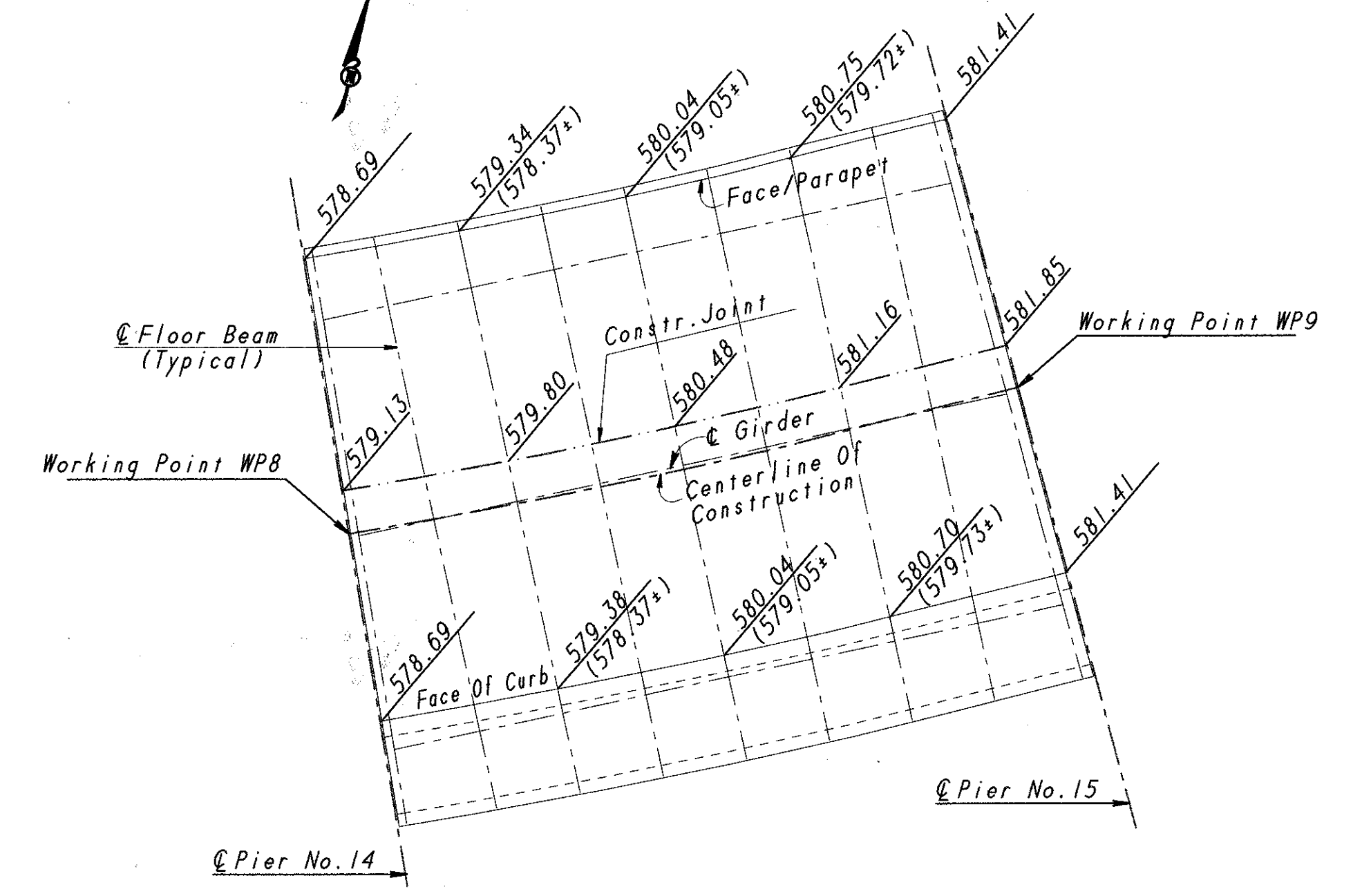
-SPM/SPN/314 Scale B

NOTE -
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along facade, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"



PLAN



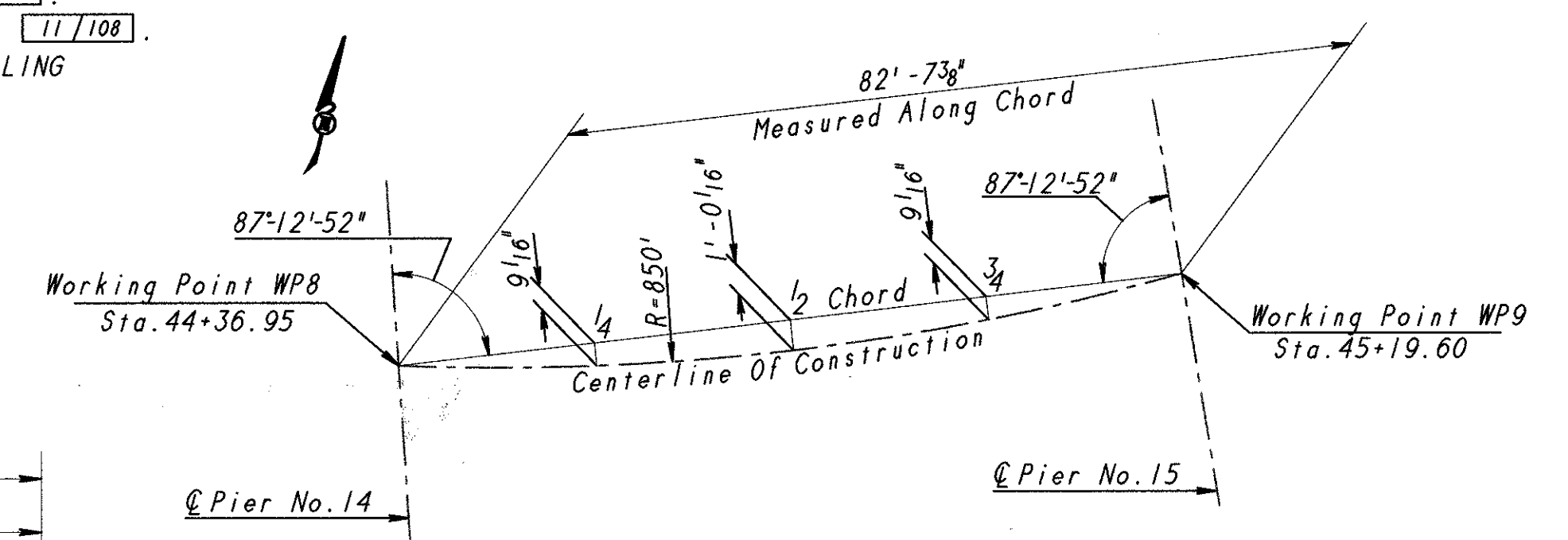
SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

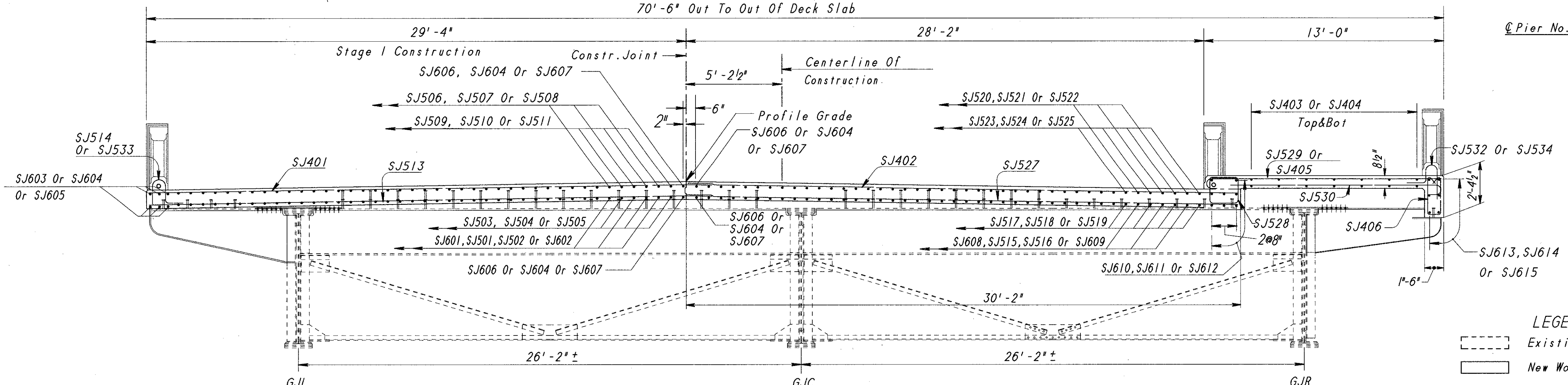
LEGEND
123.45.....Denotes Top Of Slab Elevation.
(123.45±).....Denotes Top Of Existing Steel Elevation.

NOTES

1. - Work This Sheet With General Plan Sh. 6/108.
2. - See REINFORCING STEEL BAR LIST, Sh. 96/108.
3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
5. - Work This Sheet With DECORATIVE RAILING Sh. 26/108 & 29/108.



DECK SPAN SCHEMATIC LAYOUT PLAN



SECTION JI-JI

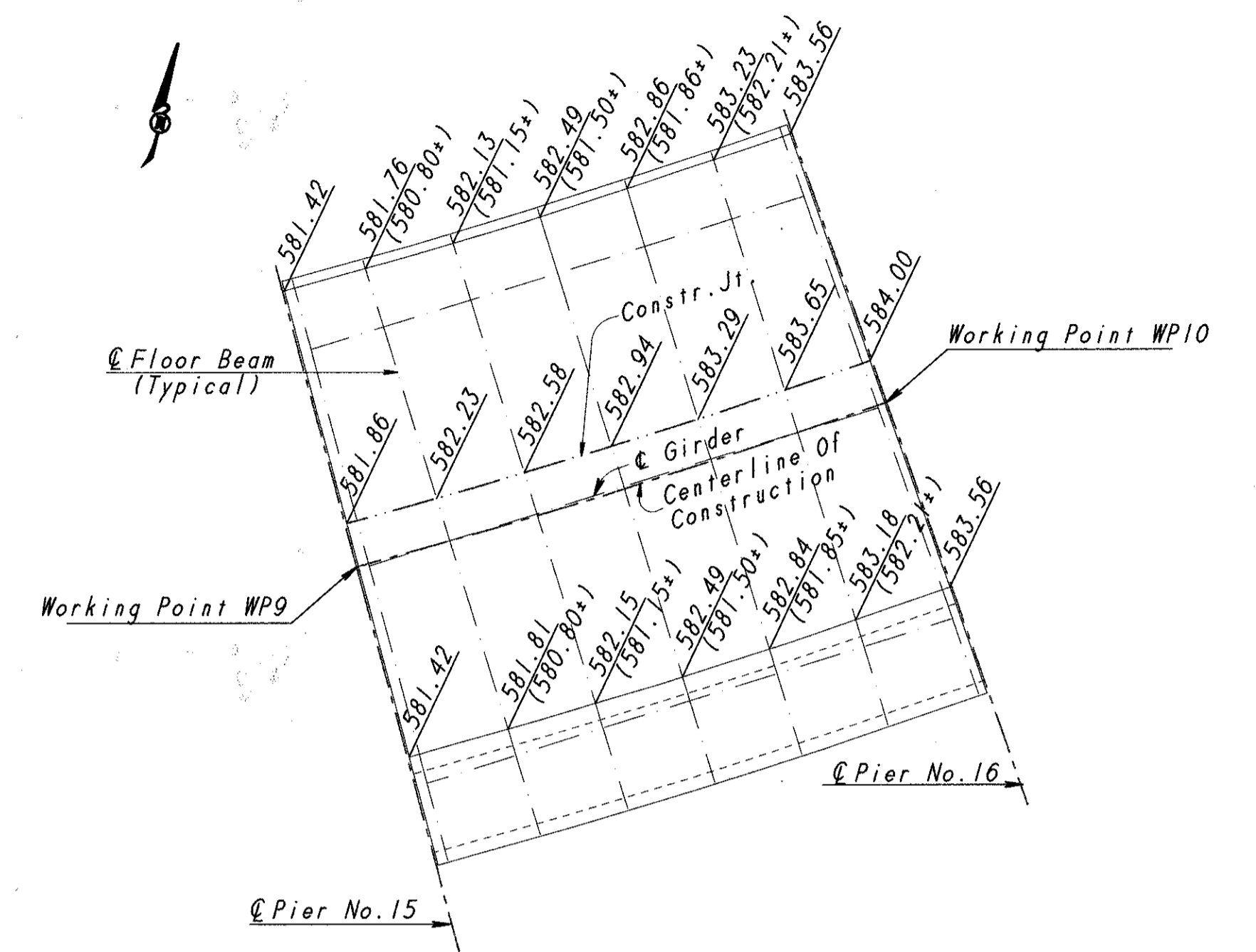
LEGEND
Existing Structure
New Work

		70/108
<p>SPAN 14-15 DECK SLAB UNIT J</p>		
<p>BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)</p>		
<p>HAMILTON COUNTY</p>		<p>Sta. 30+55.40 Sta. 47+16.19</p>
DESIGNED	DRAWN	CHECKED
ED	ED	WDD
TRACED	DATE	REVIEWED
		IEH April 96

S:\SP\1415 Scale 8

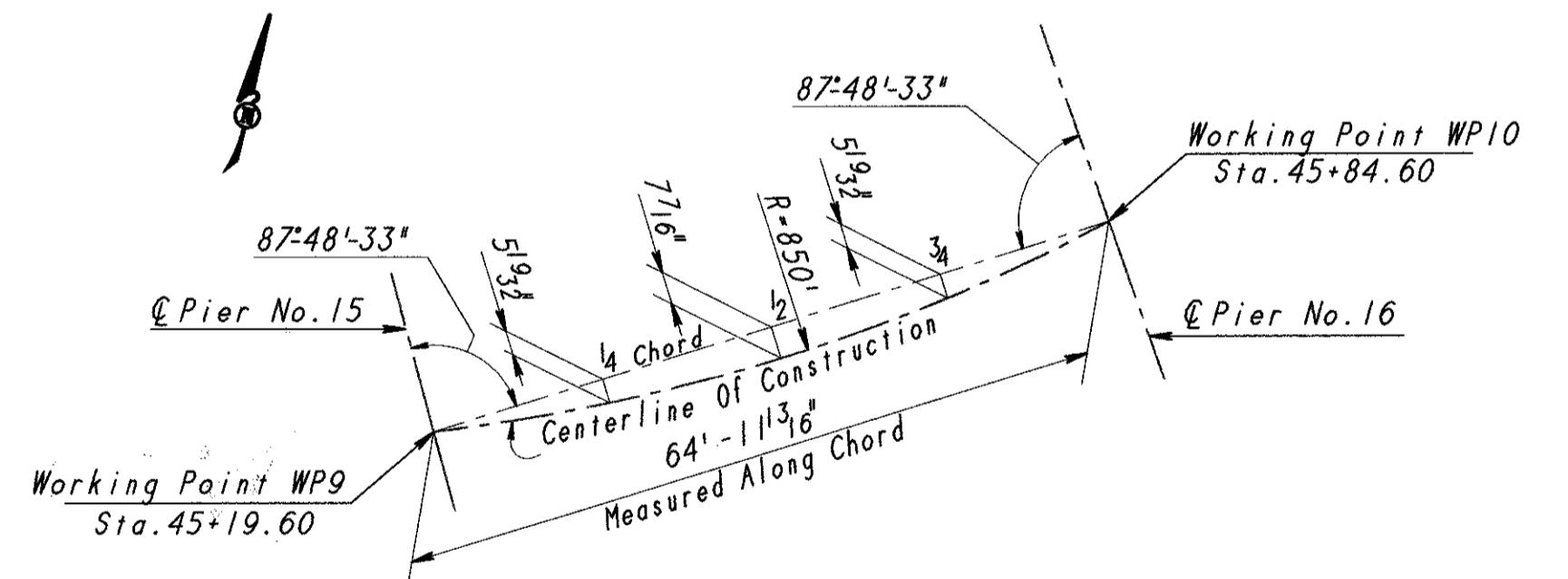
NOTE: - Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

LAP LENGTH TABLE	
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

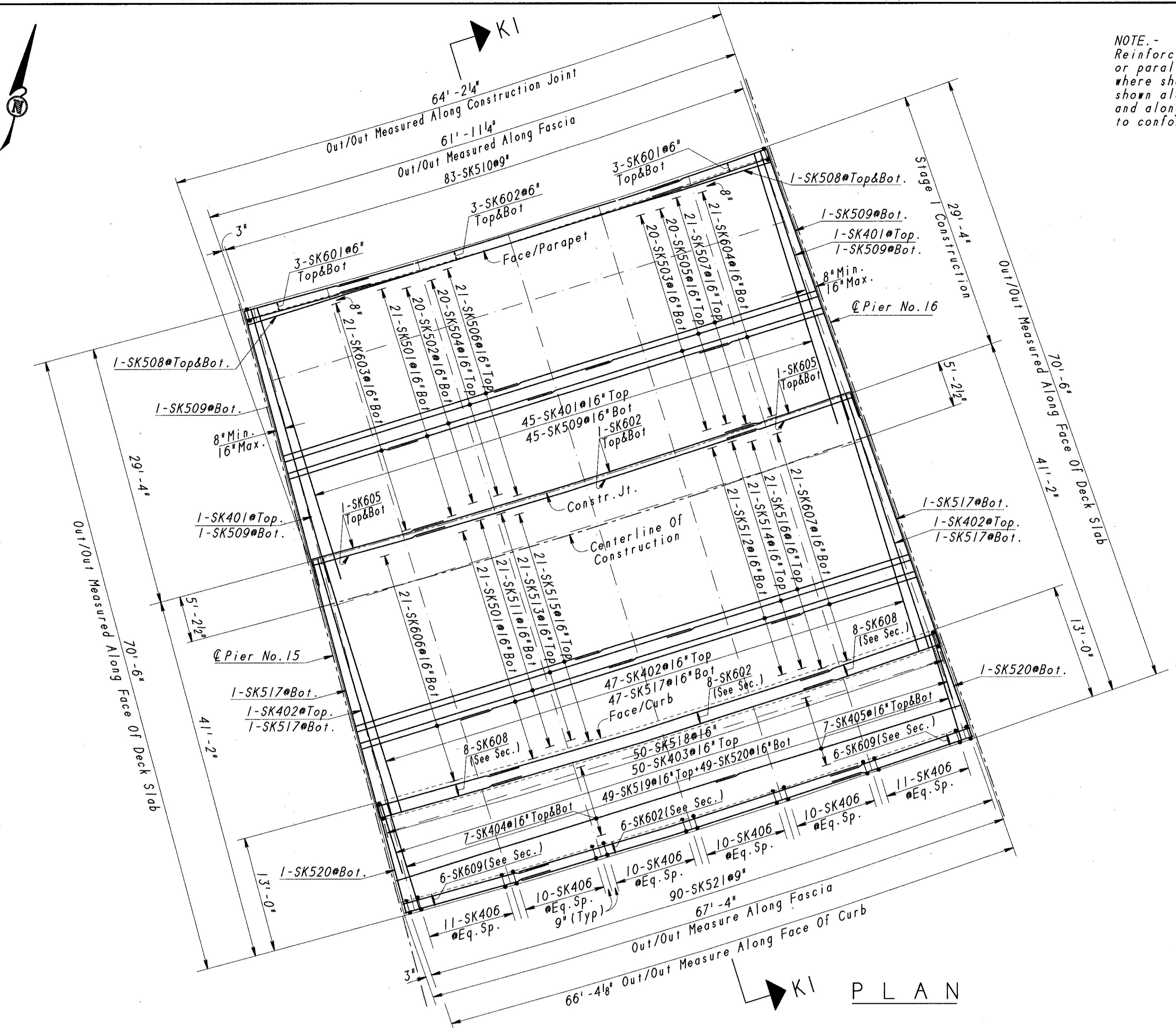


SCREED ELEVATIONS SCHEMATIC DIAGRAM
(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)
LEGEND
123.45.....Denotes Top Of Slab Elevation.
(123.45±).....Denotes Top Of Existing Steel Elevation.

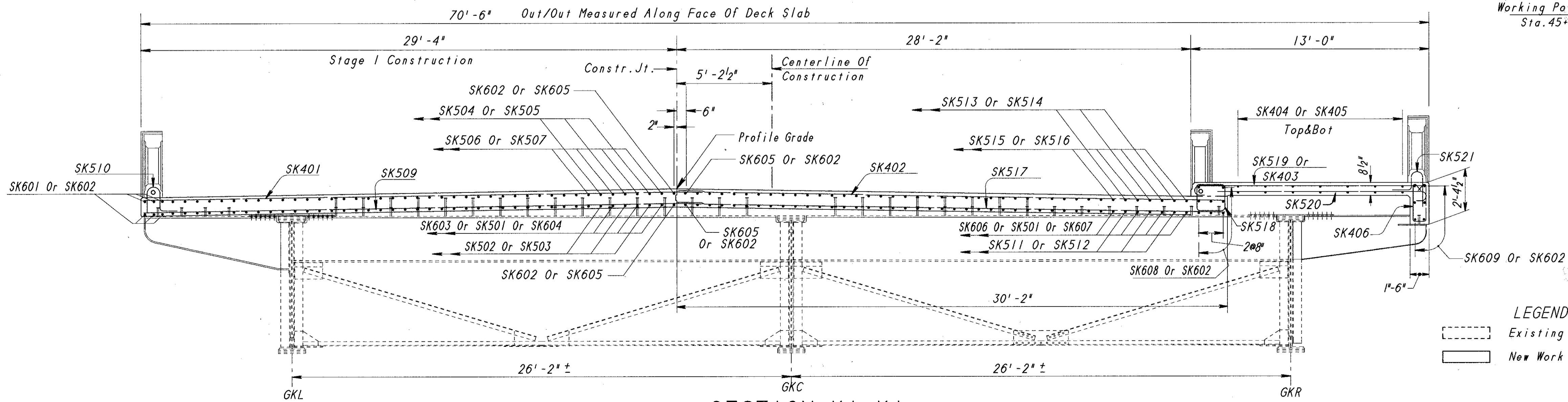
- NOTES
1. - Work This Sheet With General Plan Sh. 6/108.
 2. - See REINFORCING STEEL BAR LIST, Sh. 97/108.
 3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
 4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
 5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 80/108.



DECK SPAN SCHEMATIC LAYOUT PLAN



KI PLAN



SECTION KI-KI

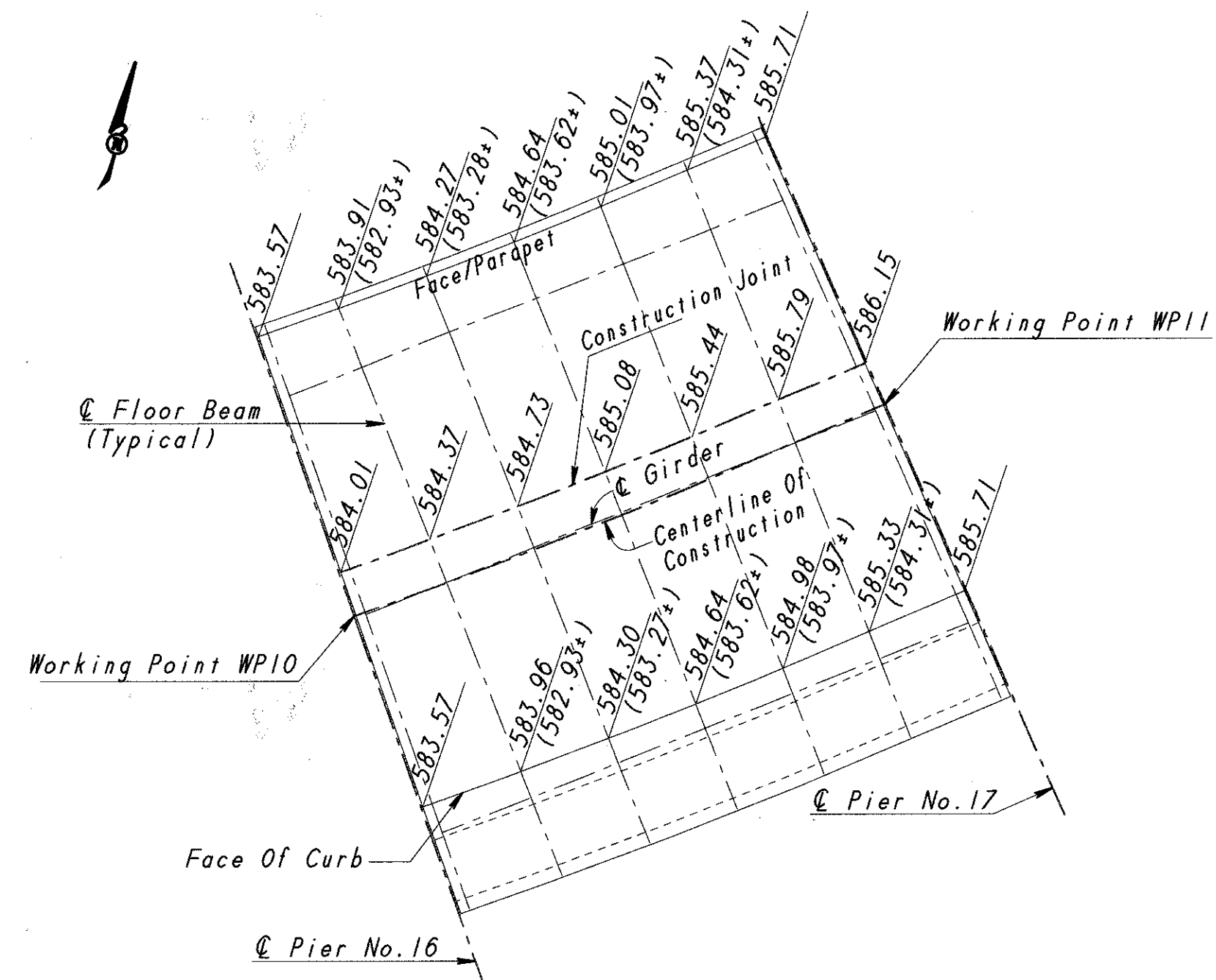
LEGEND
Existing Structure
New Work

Plum, Klausmeier & Gehrm Consultants		71/108
SPAN 15-16 DECK SLAB UNIT K		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	ED	WDD
TRACED	REVIEWED	DATE
	IEH April 96	REVISED

HAM-471-00.25

NOTE -
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

LAP LENGTH TABLE	
BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"

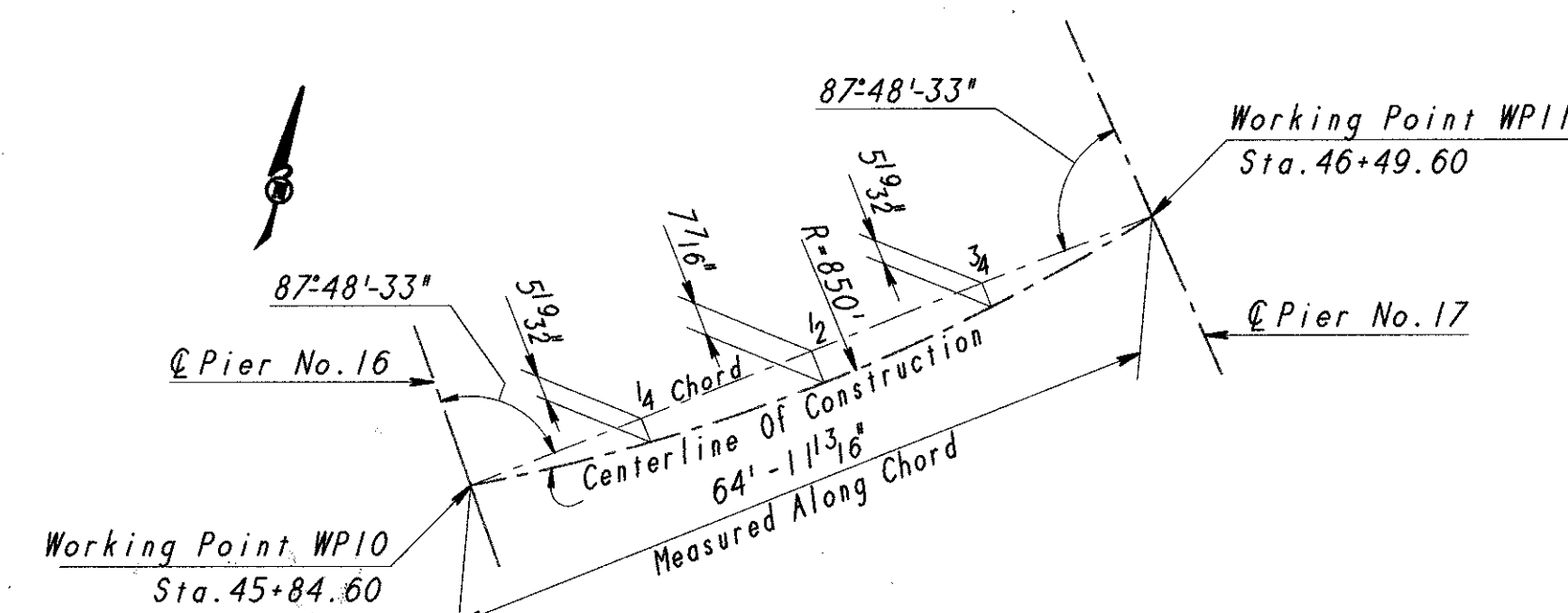


SCREED ELEVATIONS SCHEMATIC DIAGRAM
(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

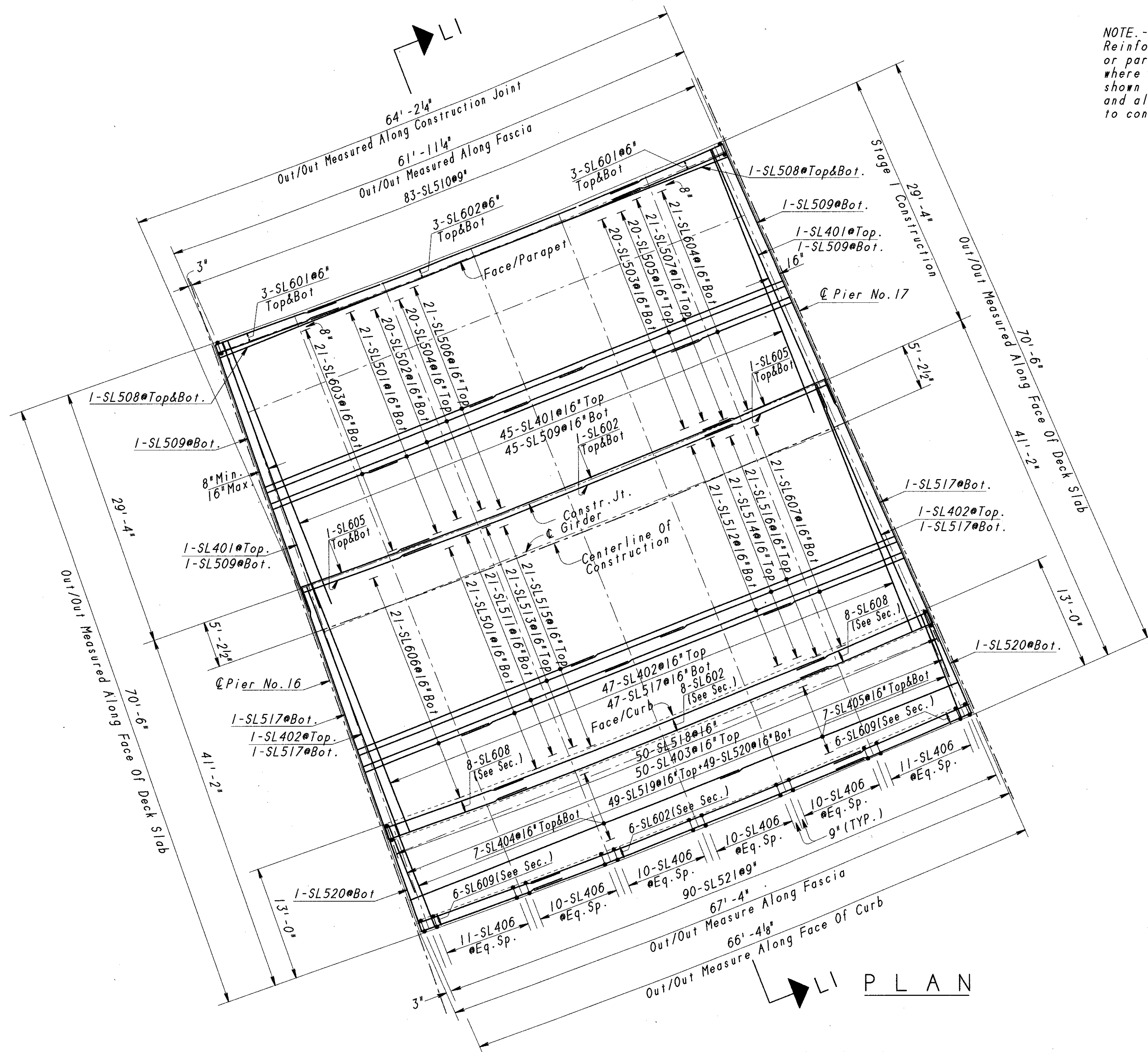
LEGEND
123.34..... Denotes Top Of Slab Elevation
(123.34)..... Denotes Top Of Existing Steel Elevation

NOTES

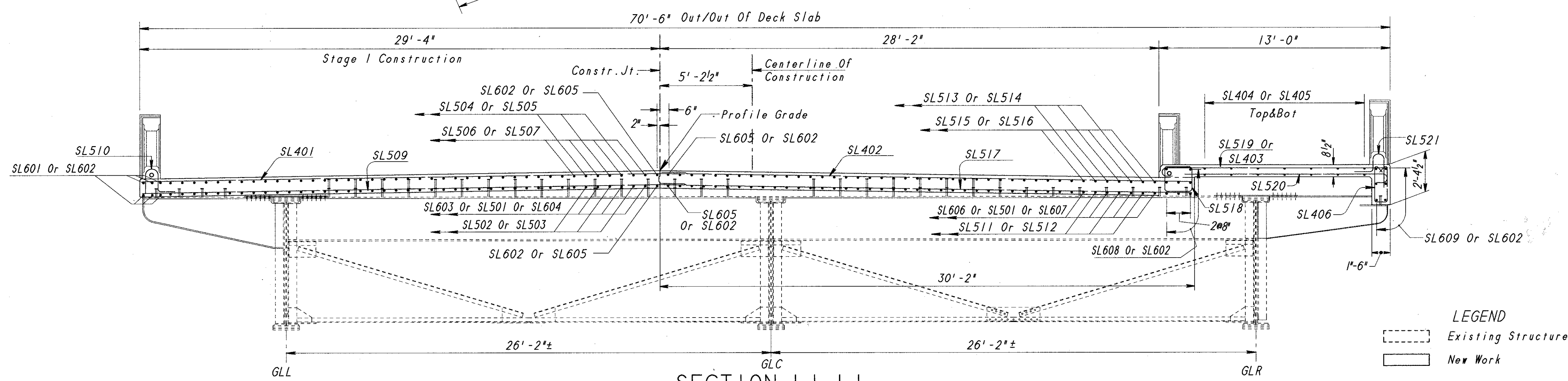
1. - Work This Sheet With General Plan Sh. 6/108.
2. - See REINFORCING STEEL BAR LIST, Sh. 97/108.
3. - See MISCELLANEOUS DETAILS, Sh. 12/108.
4. - See GENERAL NOTES, Sh. 7/108 Thru 11/108.
5. - Work This Sheet With DECORATIVE RAILING Sh. 76/108 & 80/108.



DECK SPAN SCHEMATIC LAYOUT PLAN



LI PLAN



SECTION LI-LI

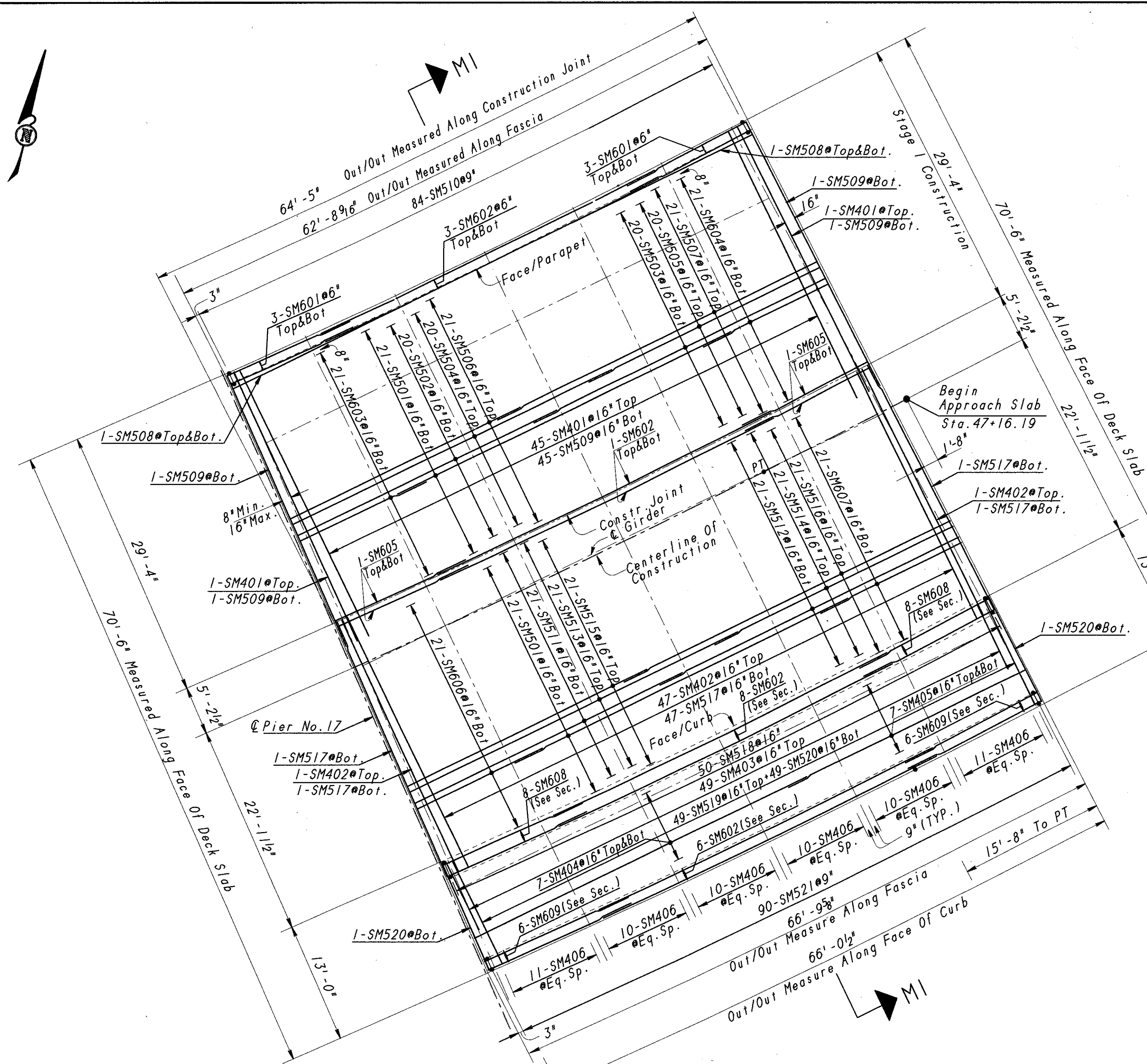
LEGEND
Existing Structure
New Work

Plum, Klausmeyer & Gehrm Consultants				72/108
SPAN 16-17 DECK SLAB UNIT L				
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY				
DESIGNED ED	DRAWN ED	TRACED WDD	CHECKED IEH	REVIEWED DATE
			April 96	REVISED
				Sta. 30+55.40 Sta. 47+16.19

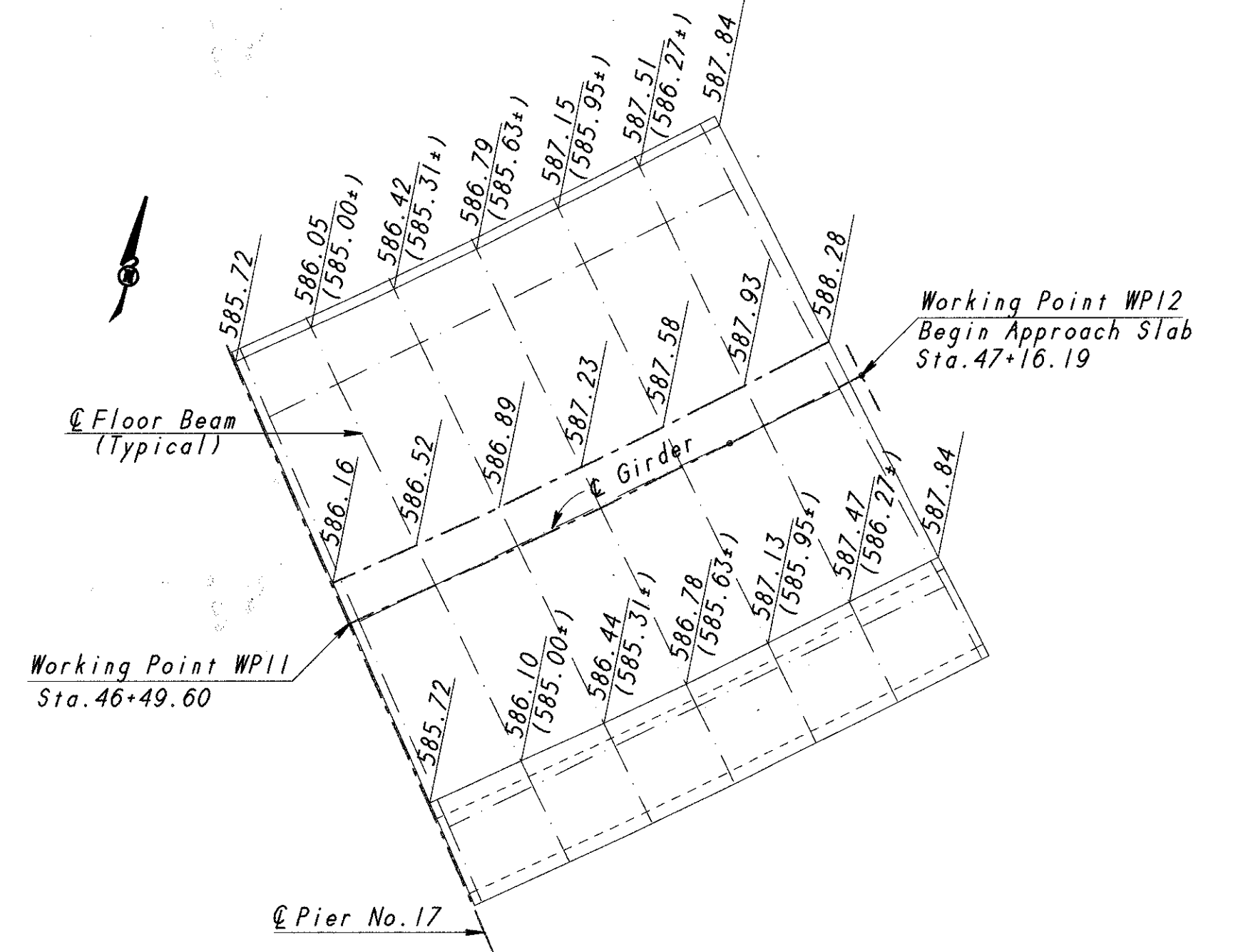
SPN16/17 SCALE 8

NOTE -
Reinforcing bars will be placed perpendicular to or parallel to the interior floor beam except where shown otherwise. Reinforcing bars shown along fasciae, along construction joint and along sidewalk curb face shall be bent on site to conform to the horizontal alignment.

BAR SIZE	LAP LENGTH
No. 4	2'-3"
No. 5	3'-3"
No. 6	3'-11"



PLAN



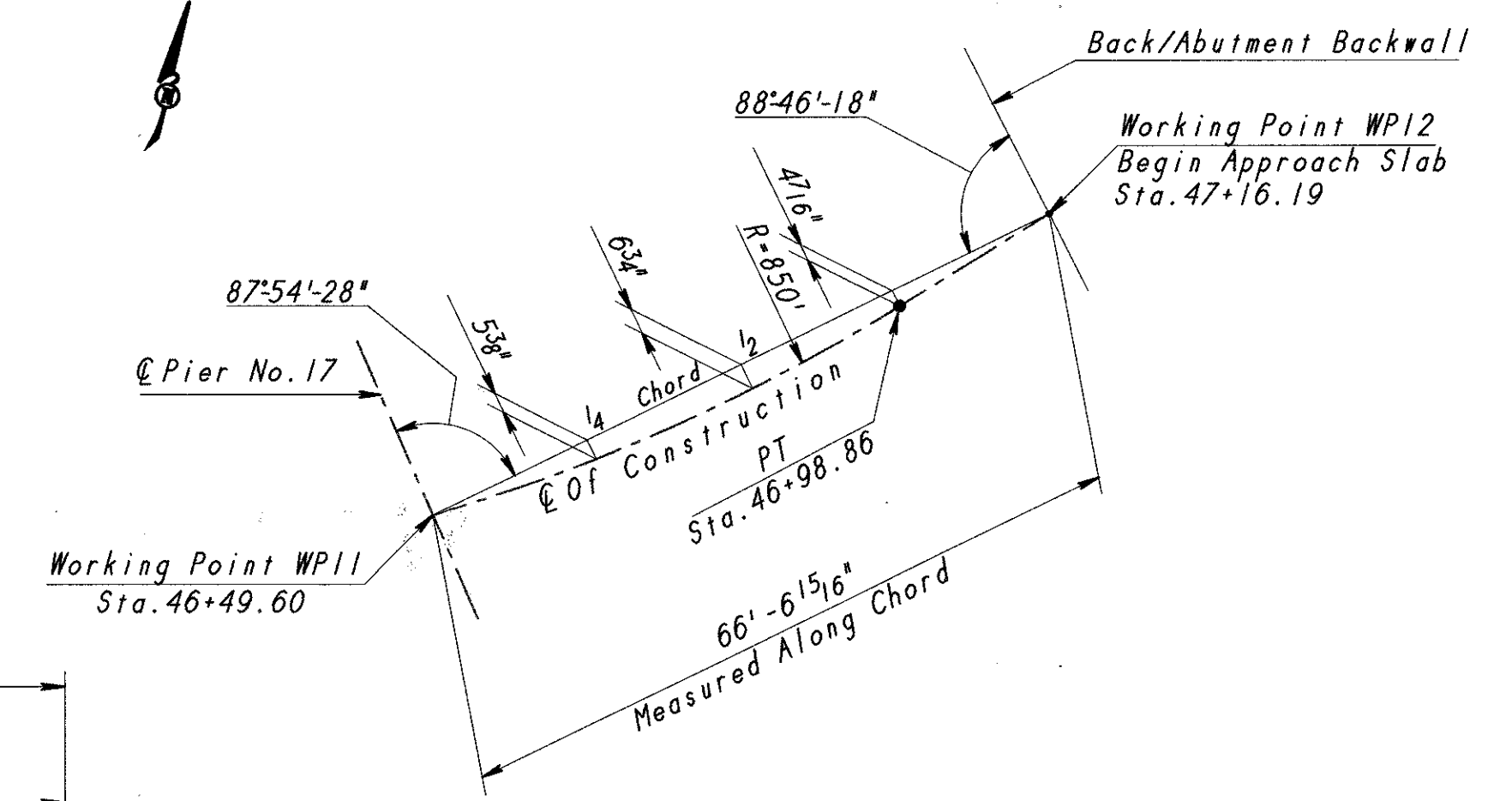
SCREED ELEVATIONS SCHEMATIC DIAGRAM

(Top Of Slab At Gutters, At Construction Joint, And Above Floor Beams As Shown, Before Concrete Is Placed.)

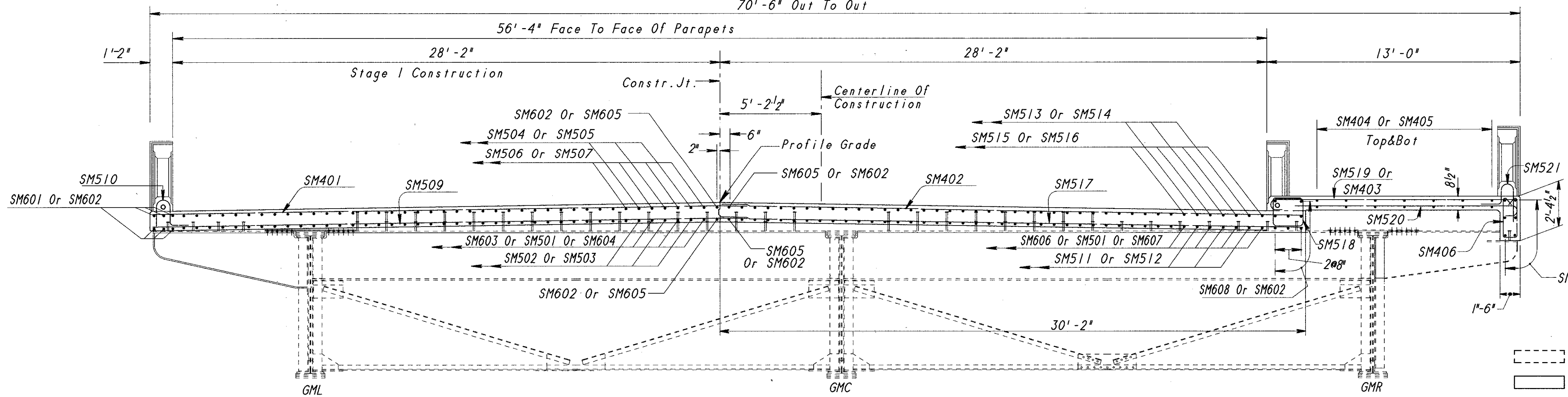
LEGEND
123.34..... Denotes Top Of Slab Elevation
(123.34).... Denotes Top Of Existing Steel Elevation

NOTES

1. - Work This Sheet With General Plan Sh. [6/108].
2. - See REINFORCING STEEL BAR LIST, Sh. [88/108].
3. - See MISCELLANEOUS DETAILS, Sh. [12/108].
4. - See GENERAL NOTES, Sh. [7/108] Thru [11/108].
5. - Work This Sheet With DECORATIVE RAILING Sh. [76/108] & [80/108].



DECK SPAN SCHEMATIC LAYOUT PLAN



SECTION MI-MI

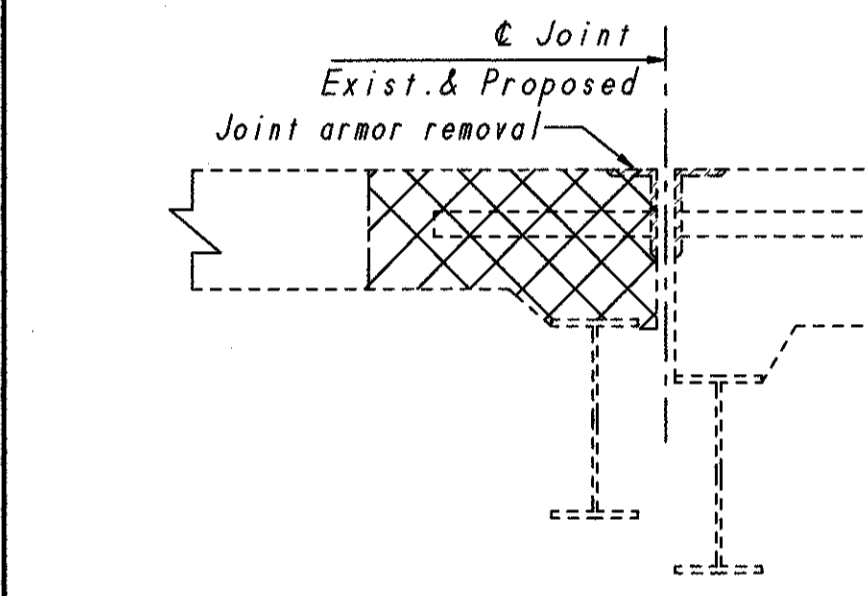
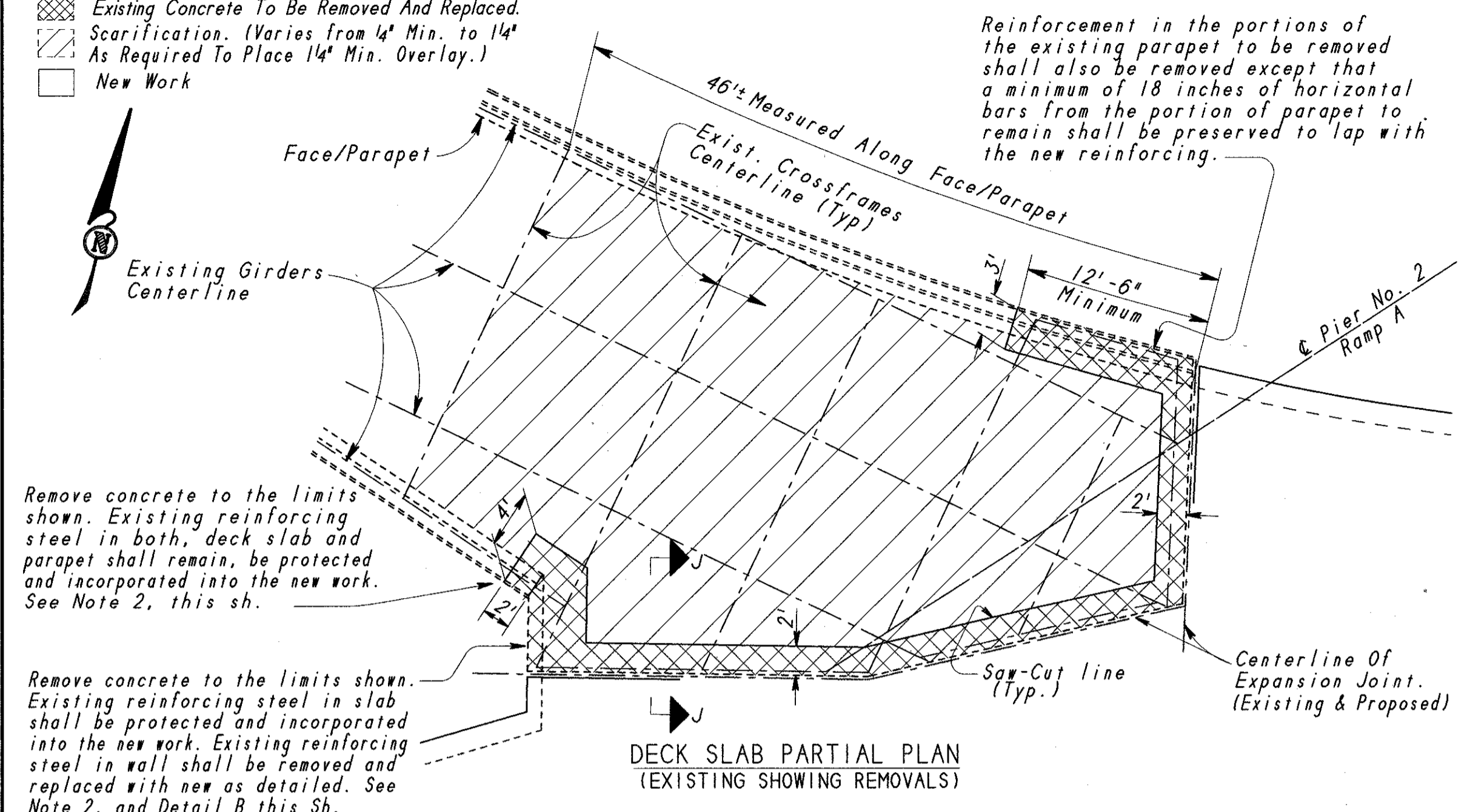
LEGEND
Existing Structure
New Work

Pflum, Klausmeyer & Gehrum Consultants		73 / 108
SPAN 17-18 DECK SLAB UNIT M		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96

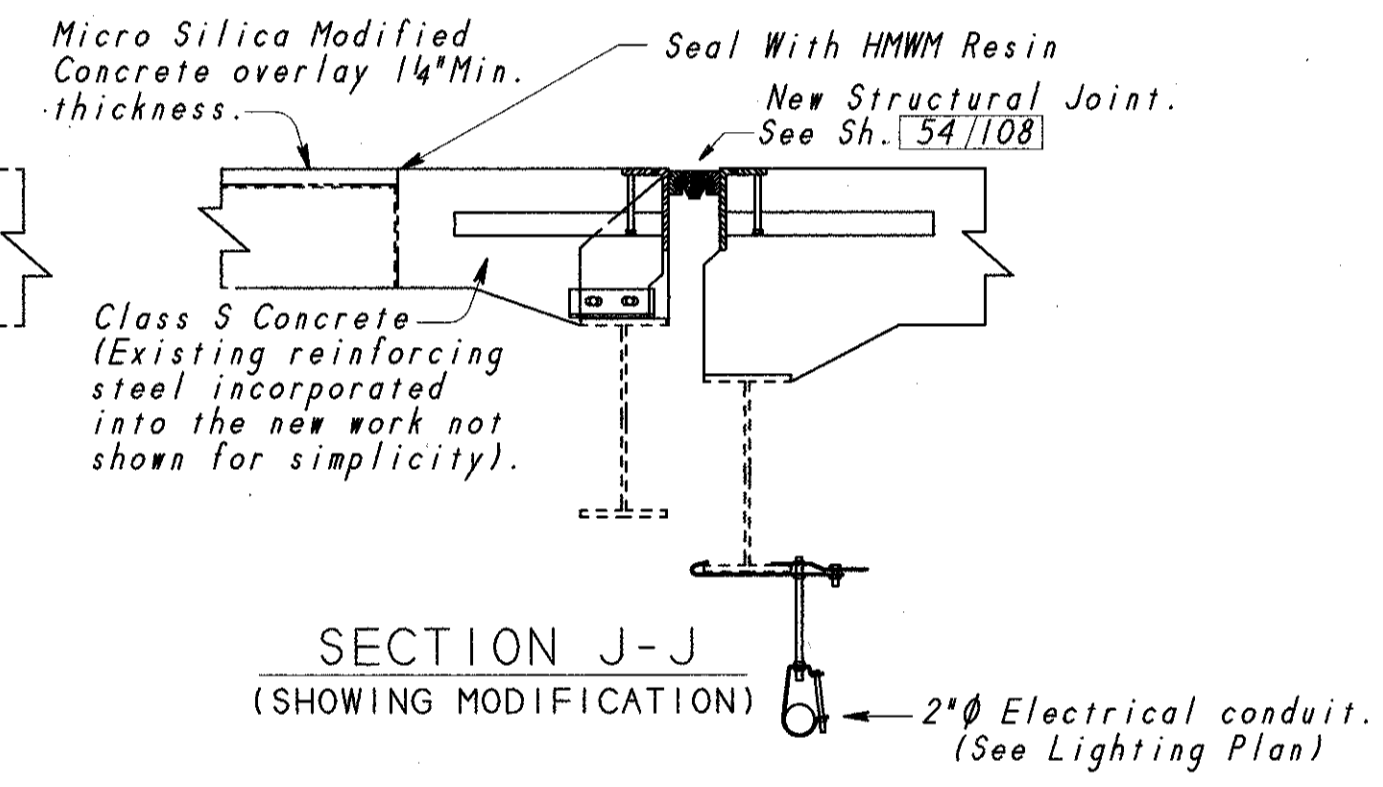
FILM SIZE: SPN1718 SCALE: 8

LEGEND

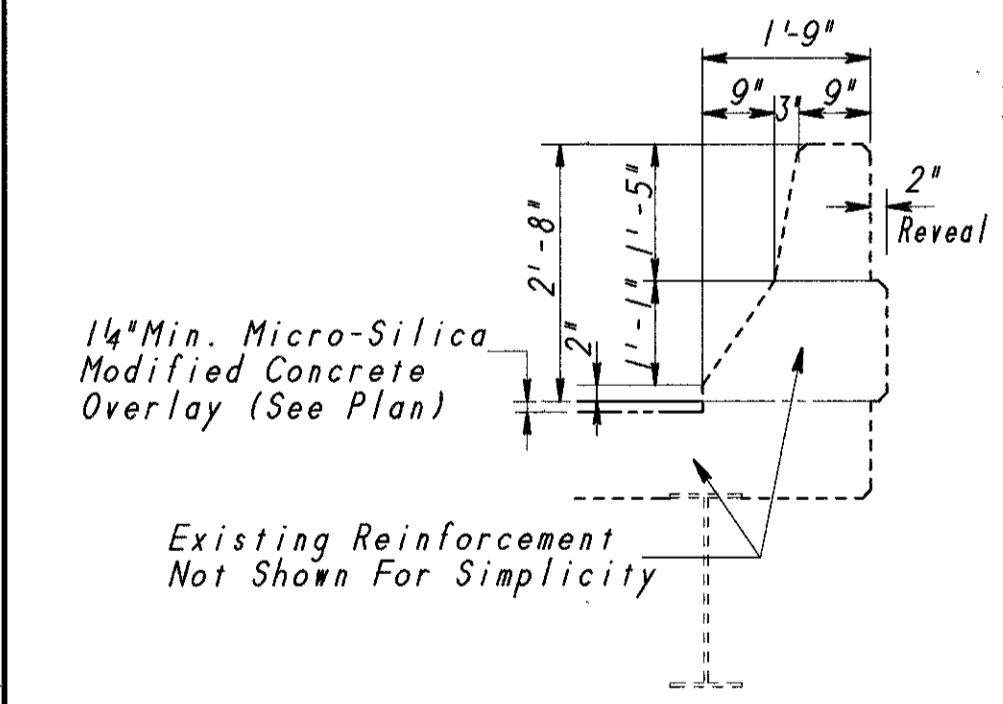
- Existing Structure
- Existing Concrete To Be Removed And Replaced.
- Scarification. (Varies from 4" Min. to 14" As Required To Place 14" Min. Overlay.)
- New Work



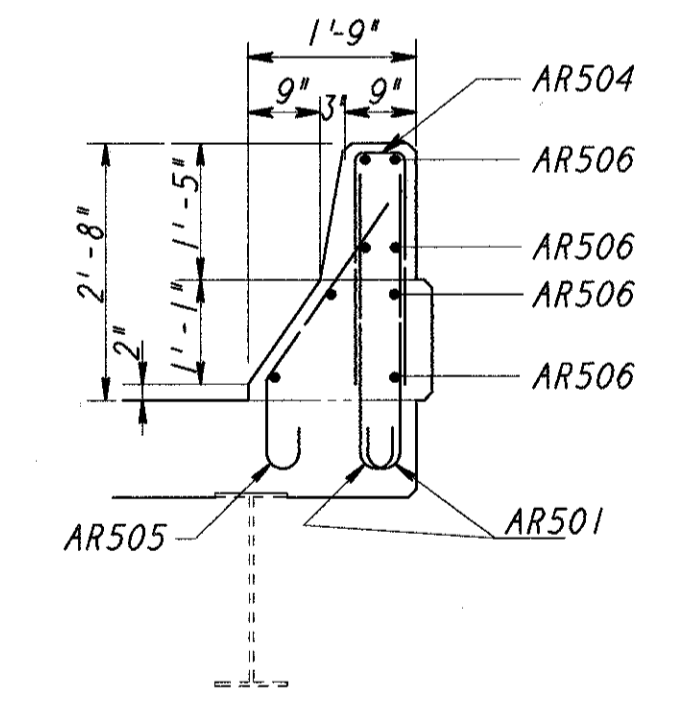
SECTION J-J
SHOWING REMOVALS



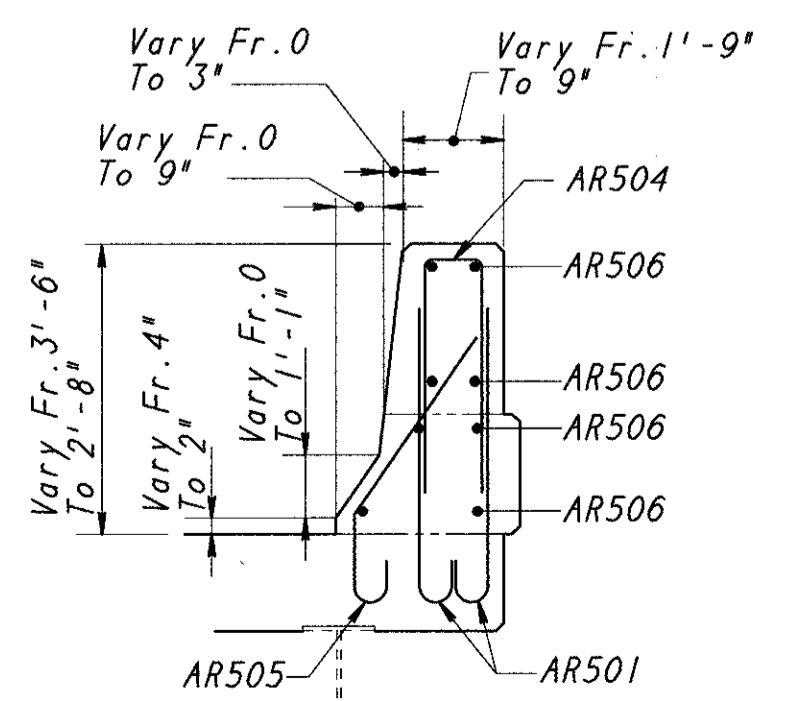
SECTION J-J
(SHOWING MODIFICATION)



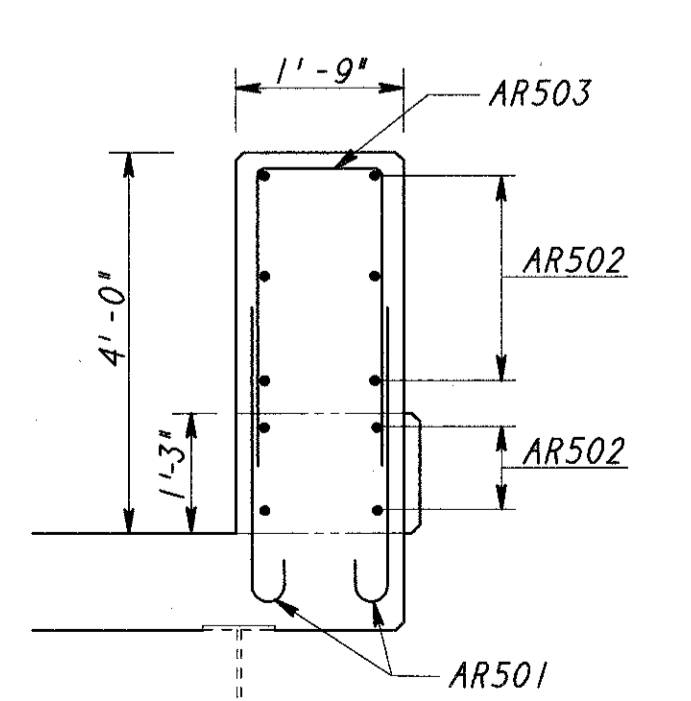
SECTION E-E



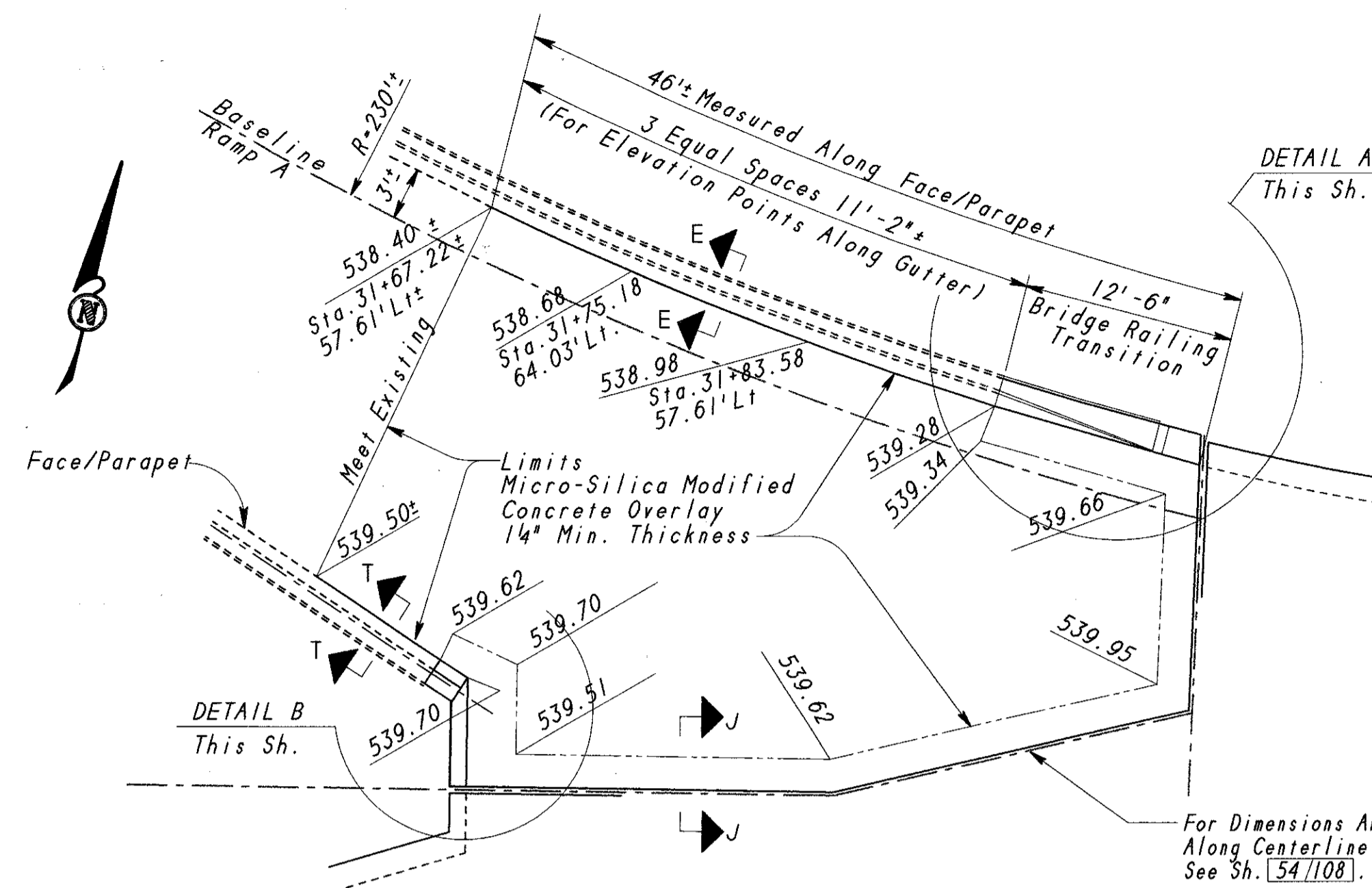
SECTION P-P



SECTION N-N



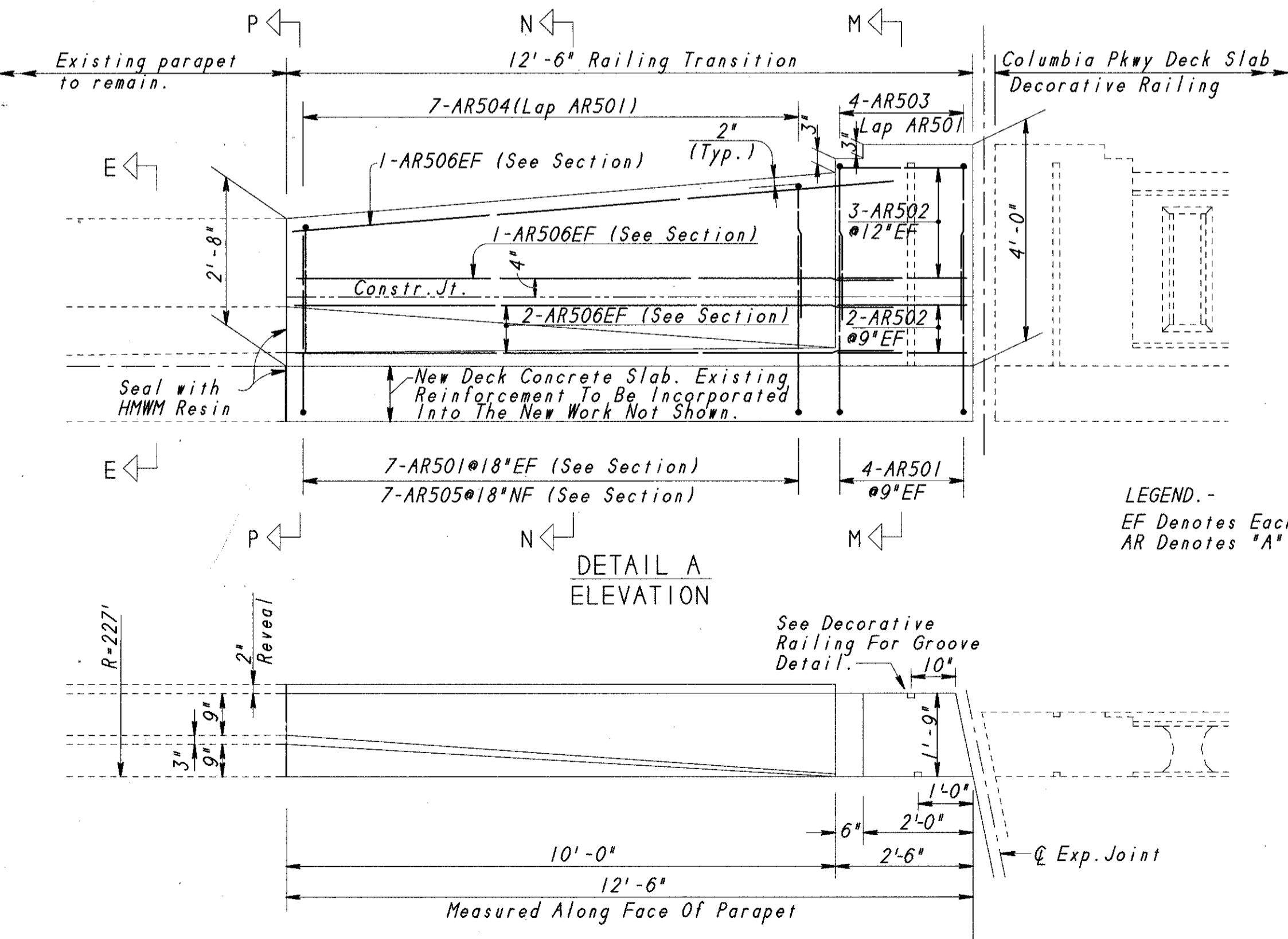
SECTION M-M



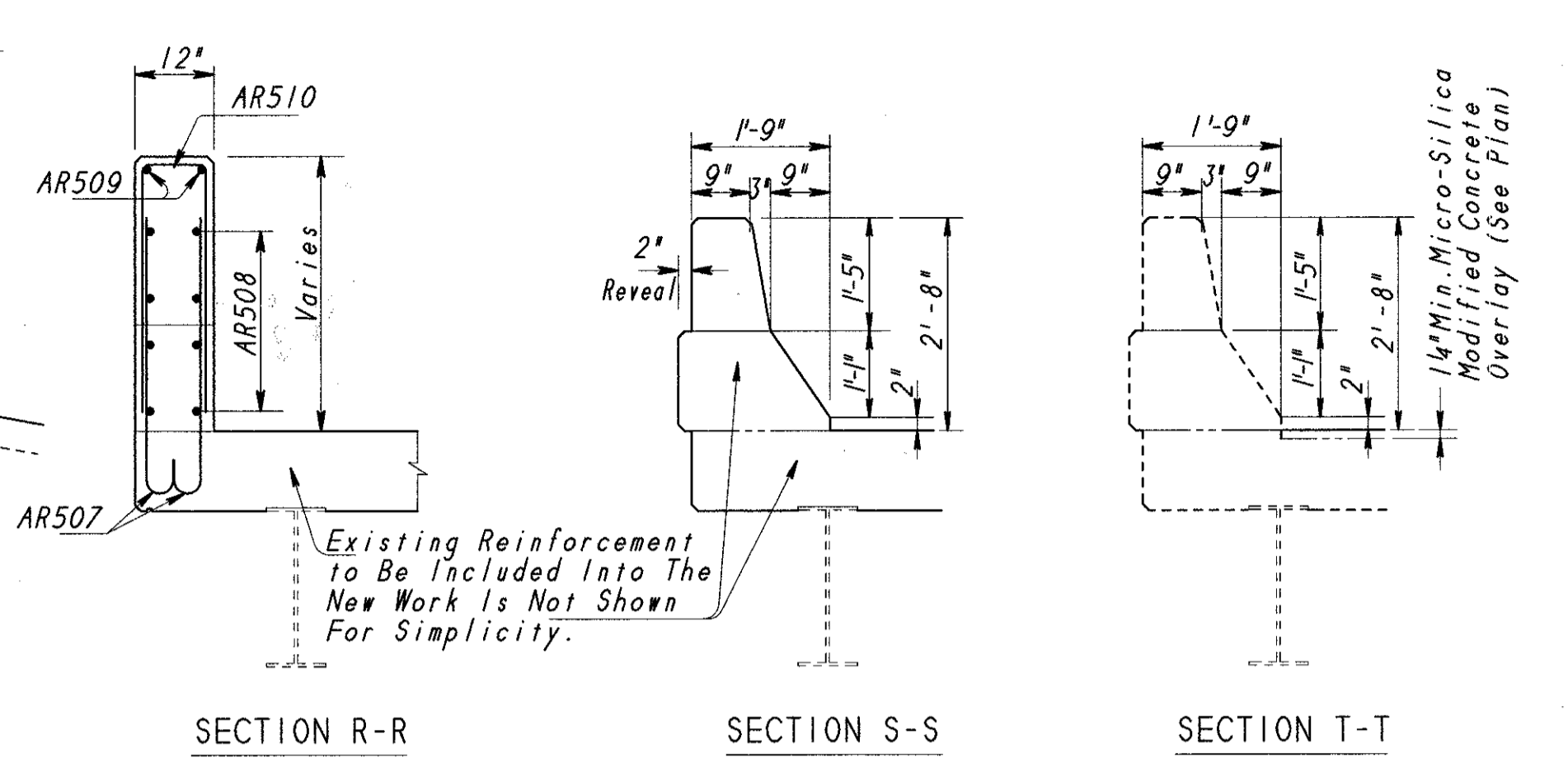
DECK SLAB PARTIAL PLAN (MODIFIED)
(ELEVATIONS SHOWN ARE FINAL TOP OF SLAB ELEVATIONS)

NOTES:

1. - All existing reinforcing steel in portions of the concrete slab to be removed and replaced shall remain, and be protected, and be incorporated into the new work.
2. - REPLACEMENT OF EXISTING REINFORCING STEEL - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 300 pounds is included in Item 509 for this purpose, listed in the "General" column of the Estimated Quantities table.
3. - Work this sheet with Deck Slab Unit B, Sh. 61/108 and with Deck Slab Unit C, Sh. 62/108.
4. - See Reinforcing Steel Bar List, Sh. 82/108
5. - See GENERAL NOTES, Sh. 7/108 Thru 11/108



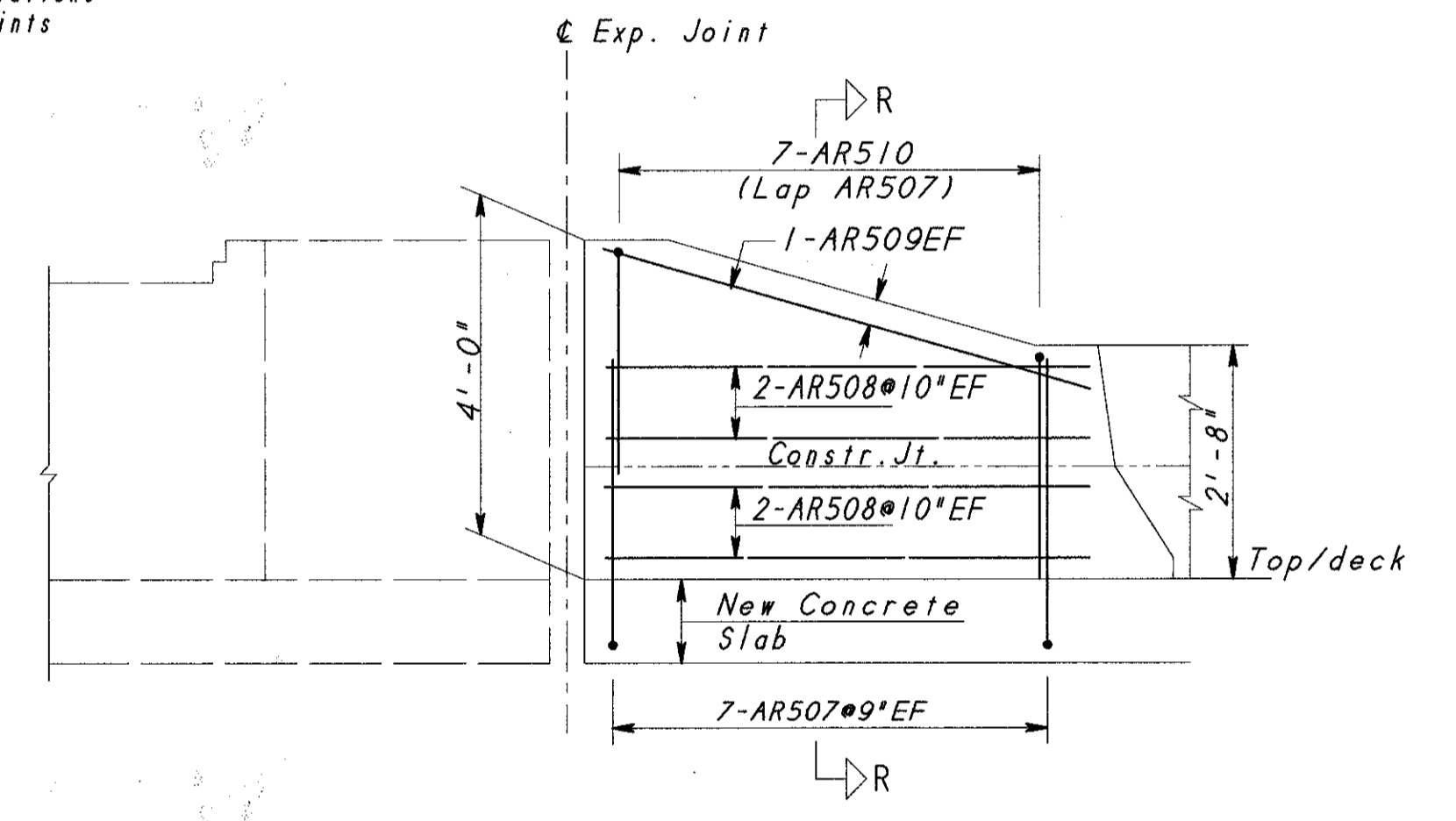
DETAIL A
ELEVATION



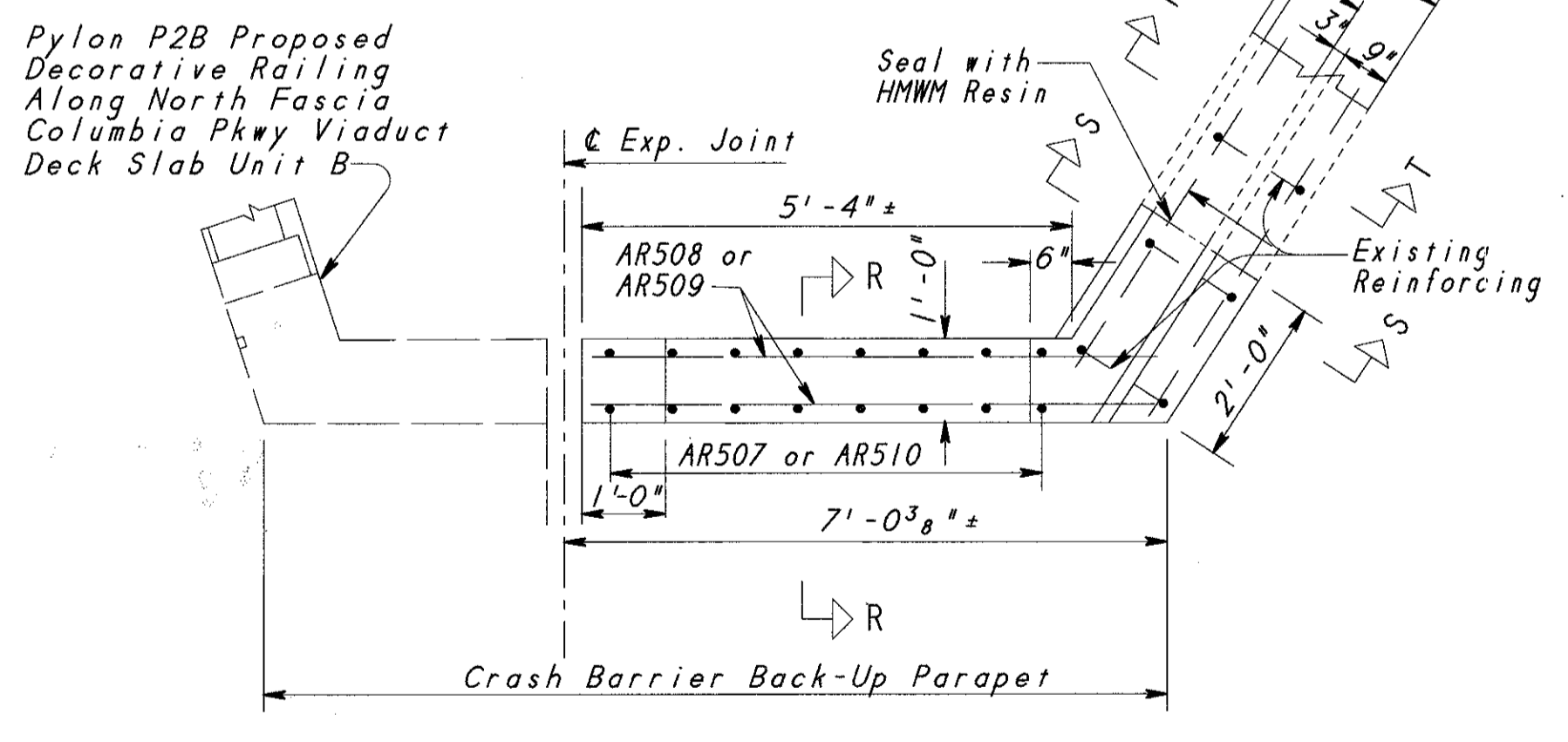
SECTION R-R

SECTION S-S

SECTION T-T



DETAIL B
ELEVATION



DETAIL B
PLAN

LEGEND -
EF Denotes Each Face
AR Denotes "A" Ramp

MODRMPA Scale 8

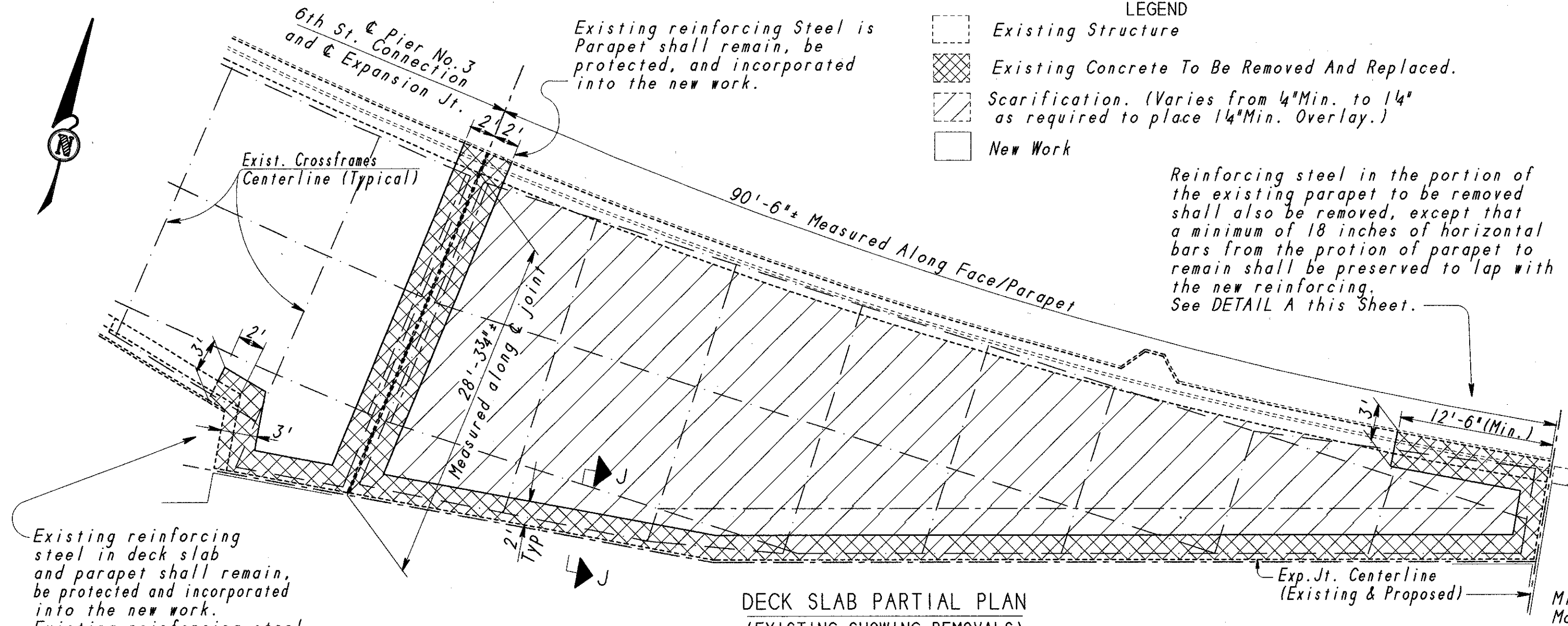
Pflum, Klausmeier & Gehrm Consultants		74 / 108
RAMP A DECK SLAB MODIFICATION		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED/PLF	CHECKED WDD
TRACED	REVIEWED IEH April 96	DATE REVISIONS

LEGEND

- Existing Structure
- Existing Concrete To Be Removed And Replaced.
- Scarification. (Varies from 1/4" Min. to 1 1/4" as required to place 1 1/4" Min. Overlay.)
- New Work

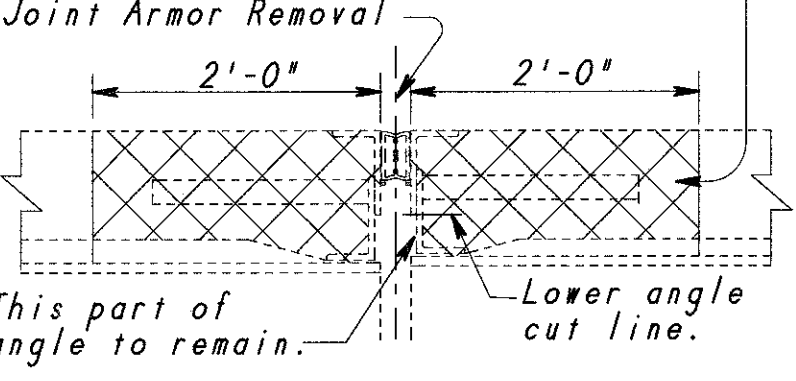
Existing reinforcing Steel is Parapet shall remain, be protected, and incorporated into the new work.

Reinforcing steel in the portion of the existing parapet to be removed shall also be removed, except that a minimum of 18 inches of horizontal bars from the portion of parapet to remain shall be preserved to lap with the new reinforcing. See DETAIL A this Sheet.

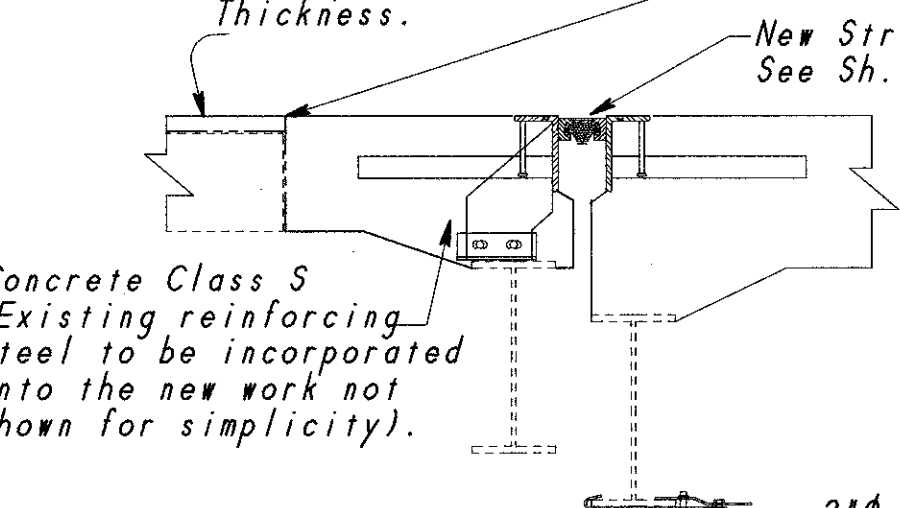
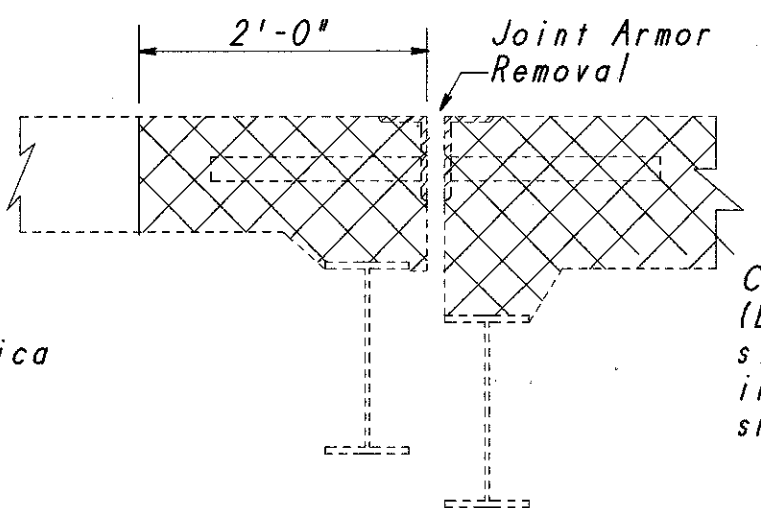
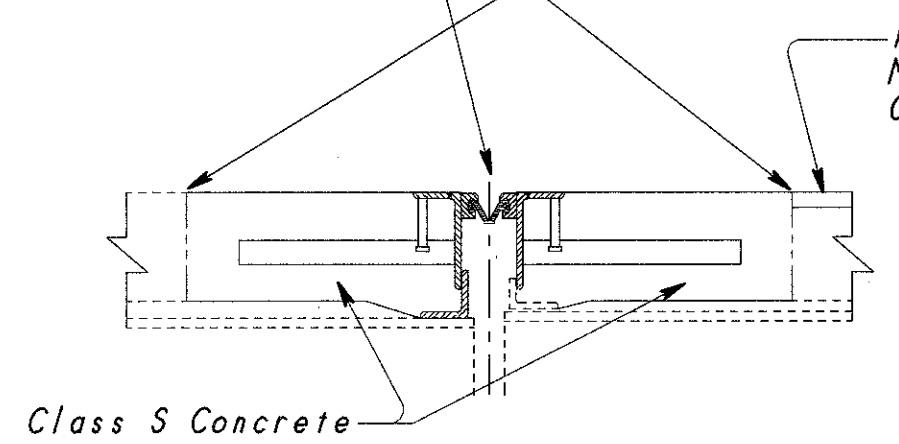


Existing reinforcing steel in deck slab and parapet shall remain, be protected and incorporated into the new work. Existing reinforcing steel in wall shall be removed and replaced with new as detailed. See Note 2 and DETAIL B This Sheet.

Removal of concrete and joint armor as shown. Existing reinforcing steel to remain be protected and incorporated into the new work.



New Structural Expansion Joint. See Sh. 57/108



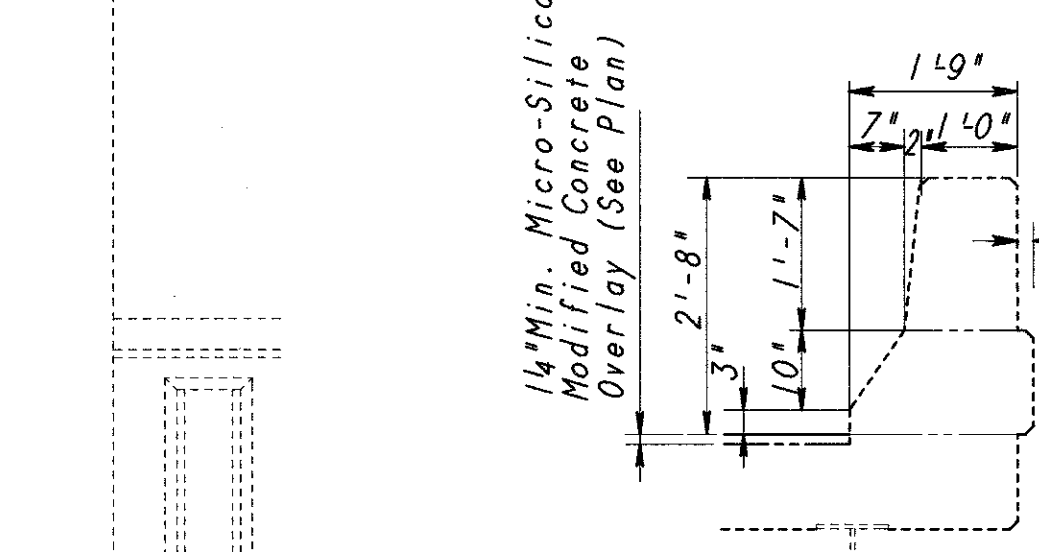
Seal with HMWM resin New Structural Joint. See Sh. 57/108

Concrete Class S (Existing reinforcing steel to be incorporated into the new work not shown for simplicity).

2" Electrical Conduit (See Lighting Plan)

SECTION J-J (SHOWING MODIFICATION)

1 1/4" Min. Micro-Silica Modified Concrete Overlay (See Plan)



SECTION E-E (All dimensions ±)

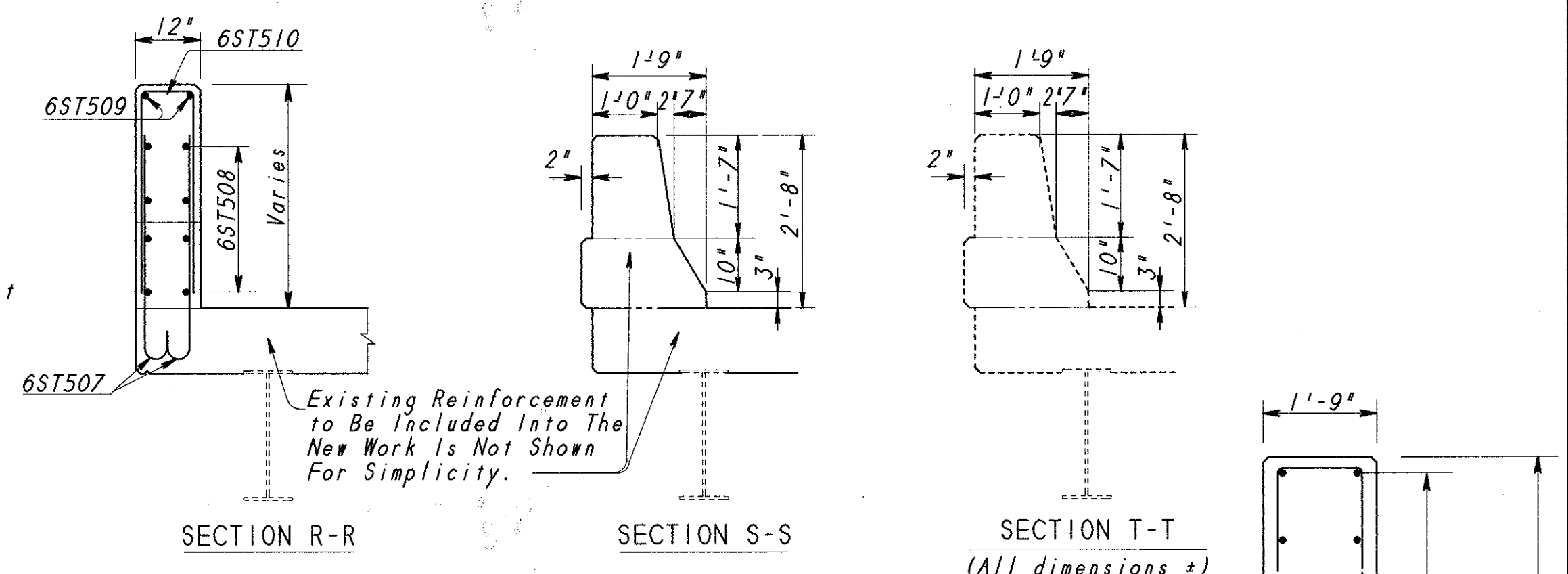
NOTES:

- All existing reinforcing steel in portions of the concrete slab to be removed and replaced shall remain, protected, and be incorporated into the new work.
- REPLACEMENT OF EXISTING REINFORCING STEEL - Any existing reinforcing bars which are to be incorporated into the new work and which are made unusable by the Contractor's removal operations shall be replaced with new steel at his cost. Any existing reinforcing bars deemed by the Engineer to be unusable because of corrosion shall be replaced with new steel. An allowance of 300 pounds is included in Item 509 for this purpose, listed in the "General" column of the Estimated Quantities table.
- Partial plan of existing structural steel frame shown is for reference only.
- Work this sheet with Deck Slab Unit G Sh. 67/108 and Deck Slab Unit H Sh. 68/108
- See Reinforcing Steel Bar List, Sh. 87/108
- See GENERAL NOTES, Sh. 7/108 thru 11/108

SECTION G-G (SHOWING REMOVALS)

SECTION G-G (SHOWING MODIFICATION)

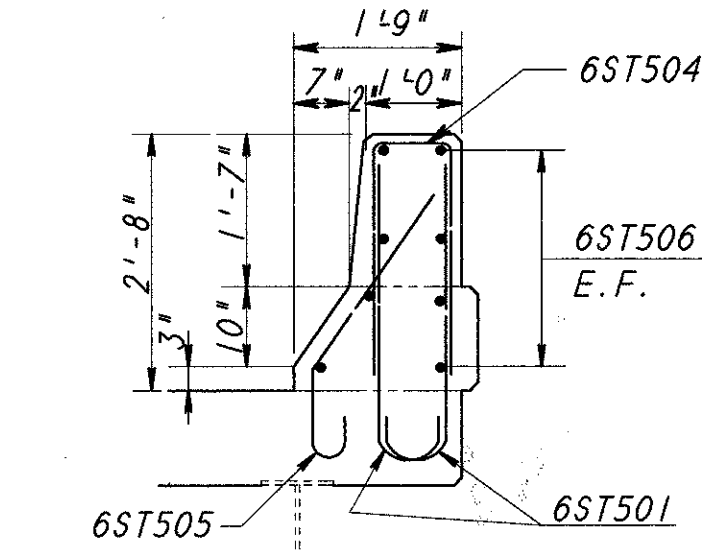
DECK SLAB PARTIAL PLAN (MODIFIED) (ELEVATIONS SHOWN ARE FINAL TOP OF SLAB ELEVATIONS)



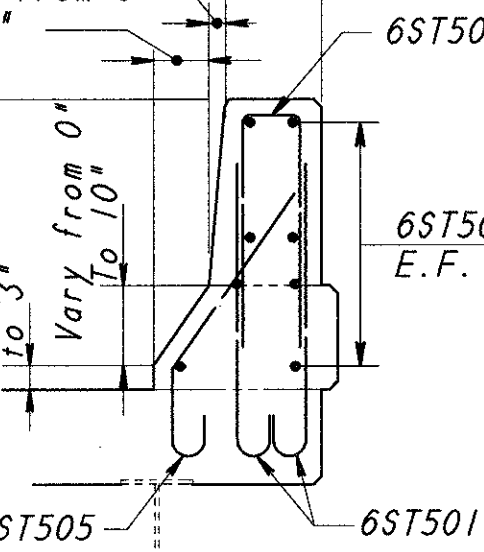
SECTION R-R

SECTION S-S

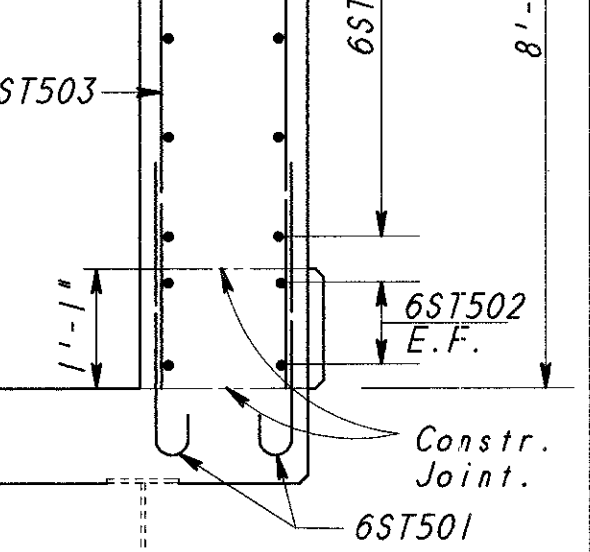
SECTION T-T (All dimensions ±)



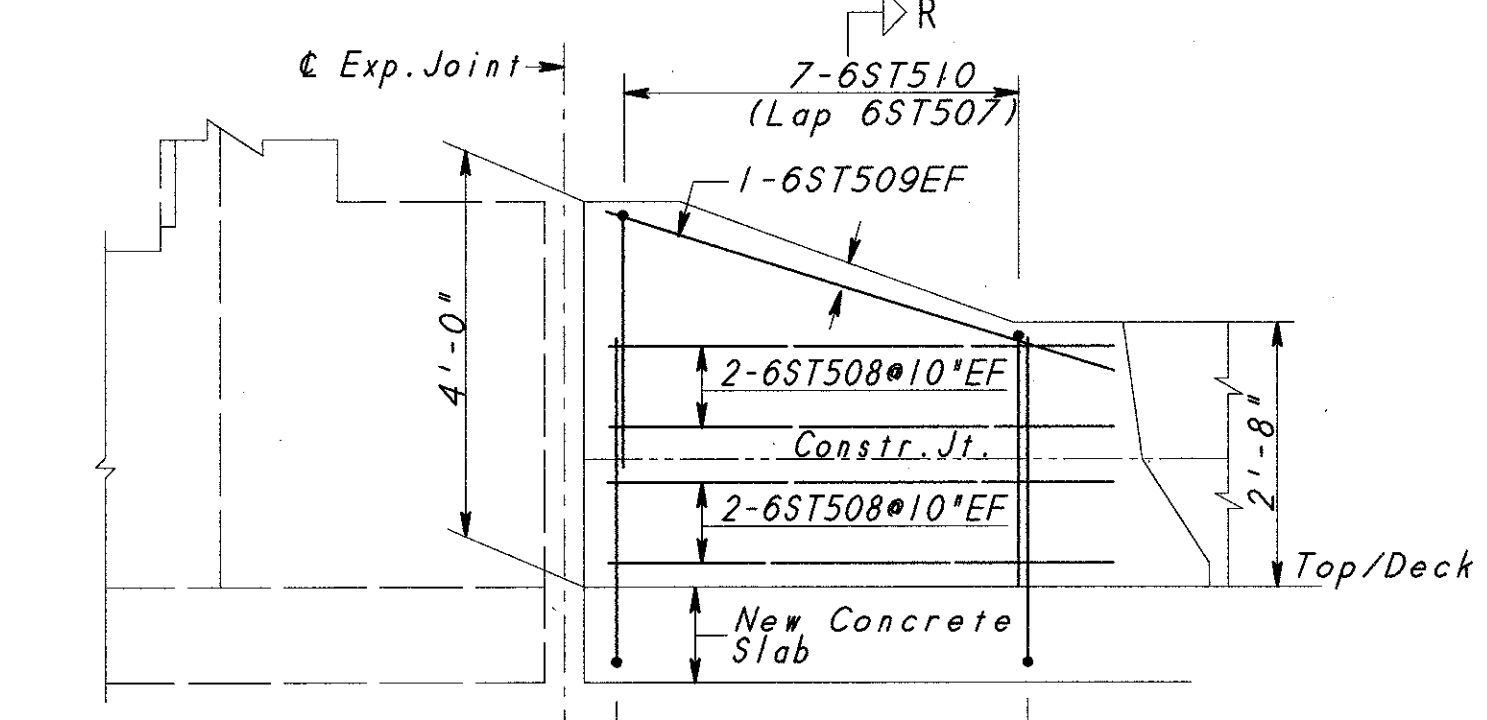
SECTION P-P



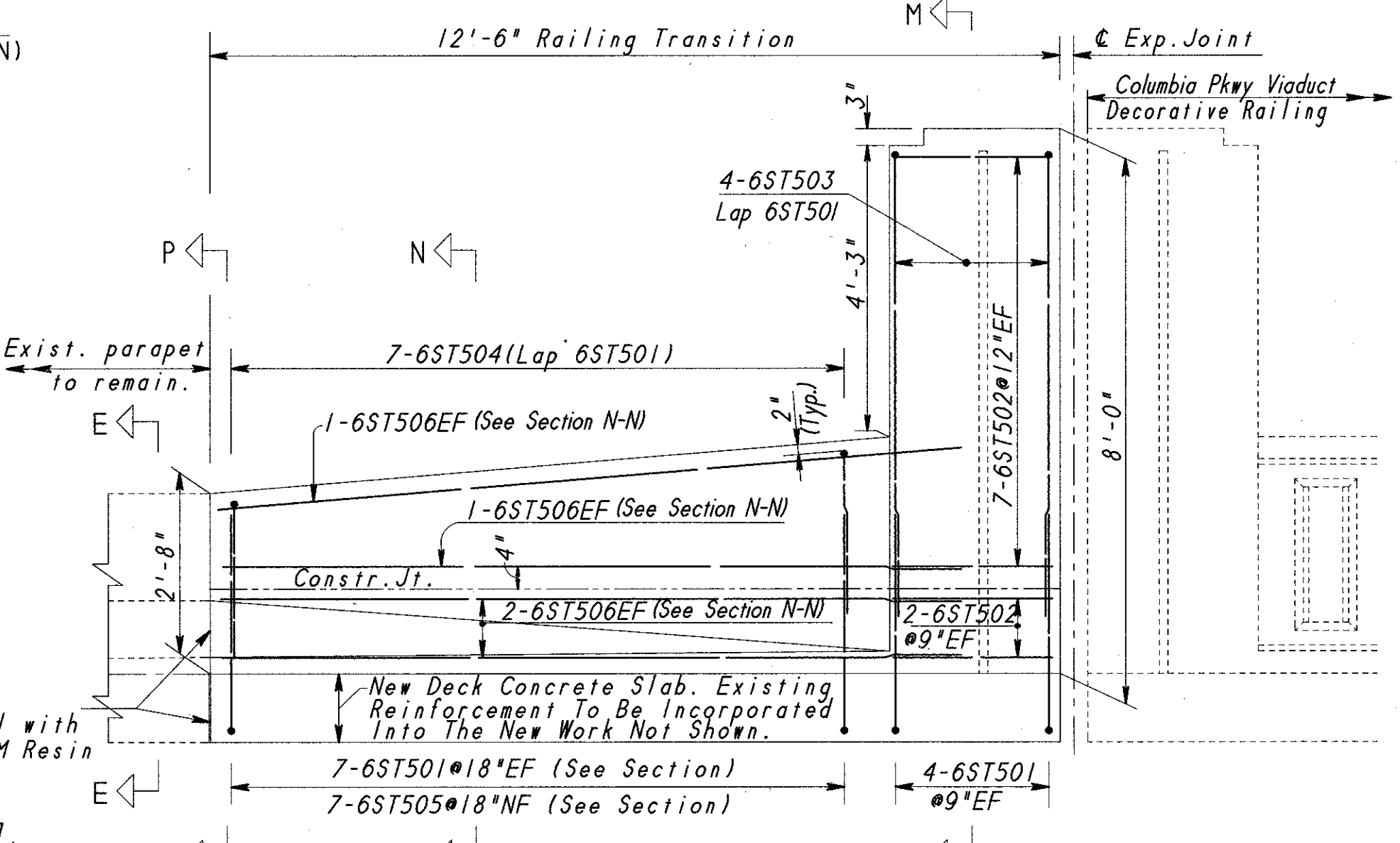
SECTION N-N



SECTION M-M

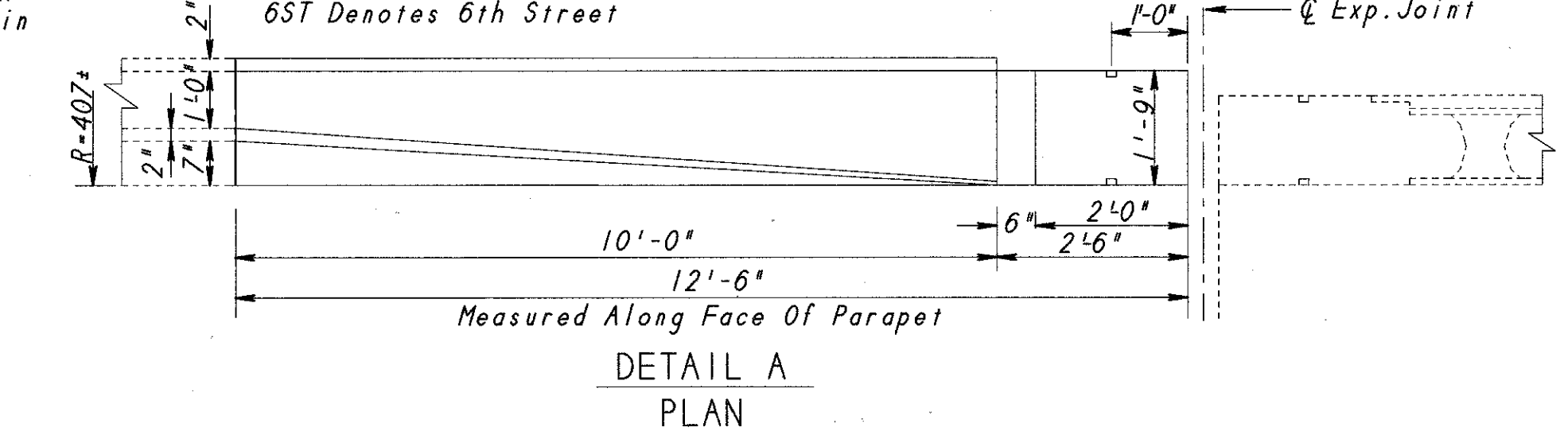


DETAIL B ELEVATION B-B



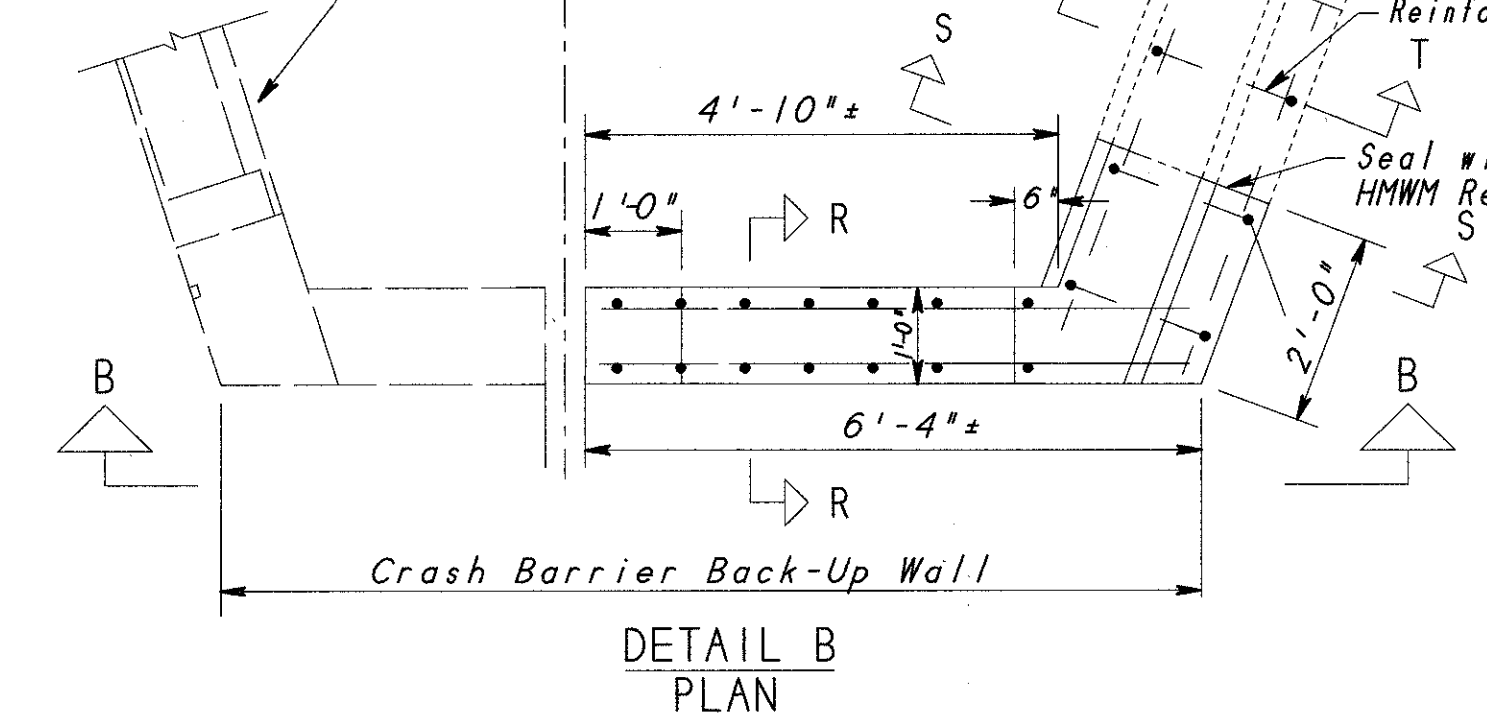
DETAIL A ELEVATION

LEGEND
 EF Denotes Each Face
 6ST Denotes 6th Street



DETAIL A PLAN

Pylon P2G Proposed Decorative Railing Along North Fascia Columbia Pkwy Viaduct Deck Slab Unit G



DETAIL B PLAN

Pilum, Klausmeyer & Gehrum
 CONSULTANTS

SIXTH STREET CONNECTION
 BRIDGE No. HAM-471-0025
 (COLUMBIA PARKWAY VIADUCT OVER I-471)

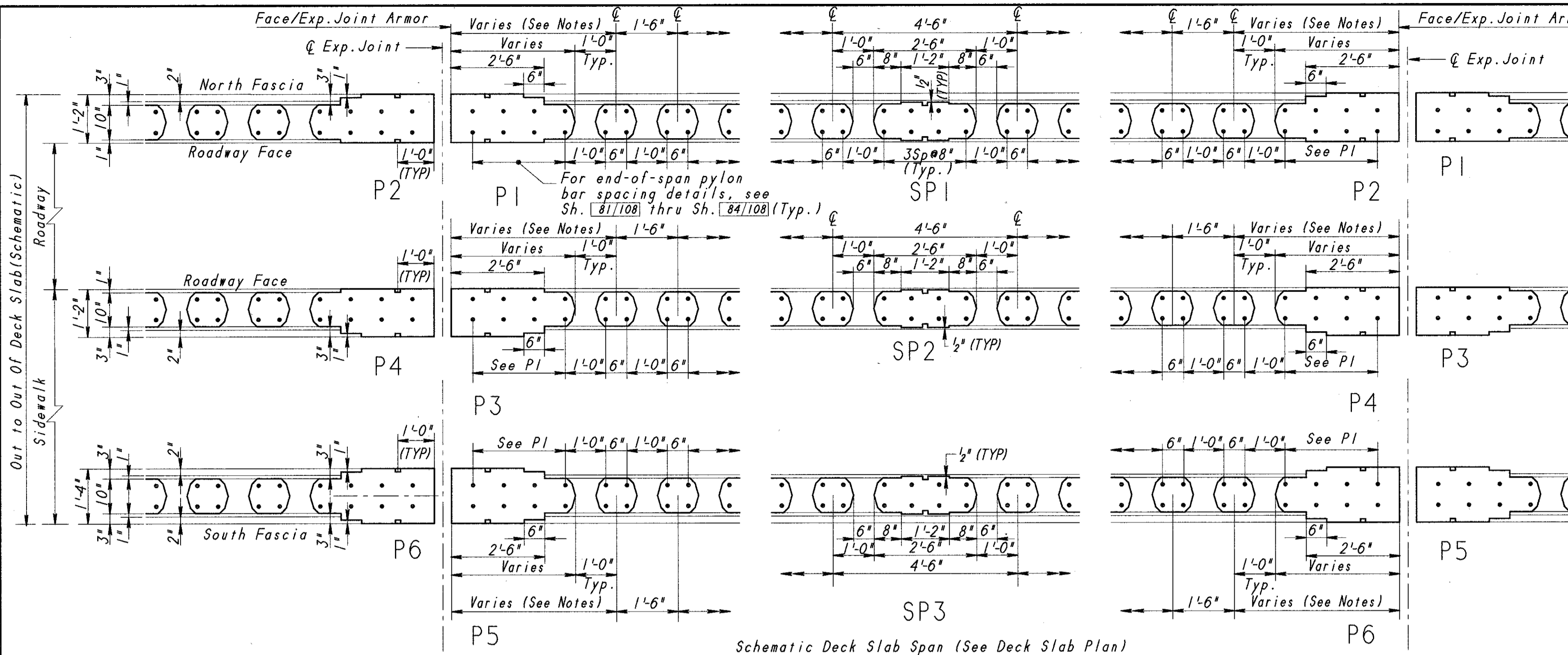
HAMILTON COUNTY	DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISION
	ED	ED/PLF	WDD	IEH	April 96		

Sta. 30+55.40
 Sta. 47+16.19

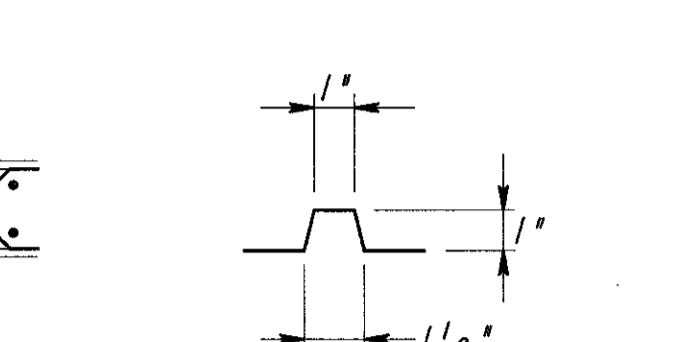
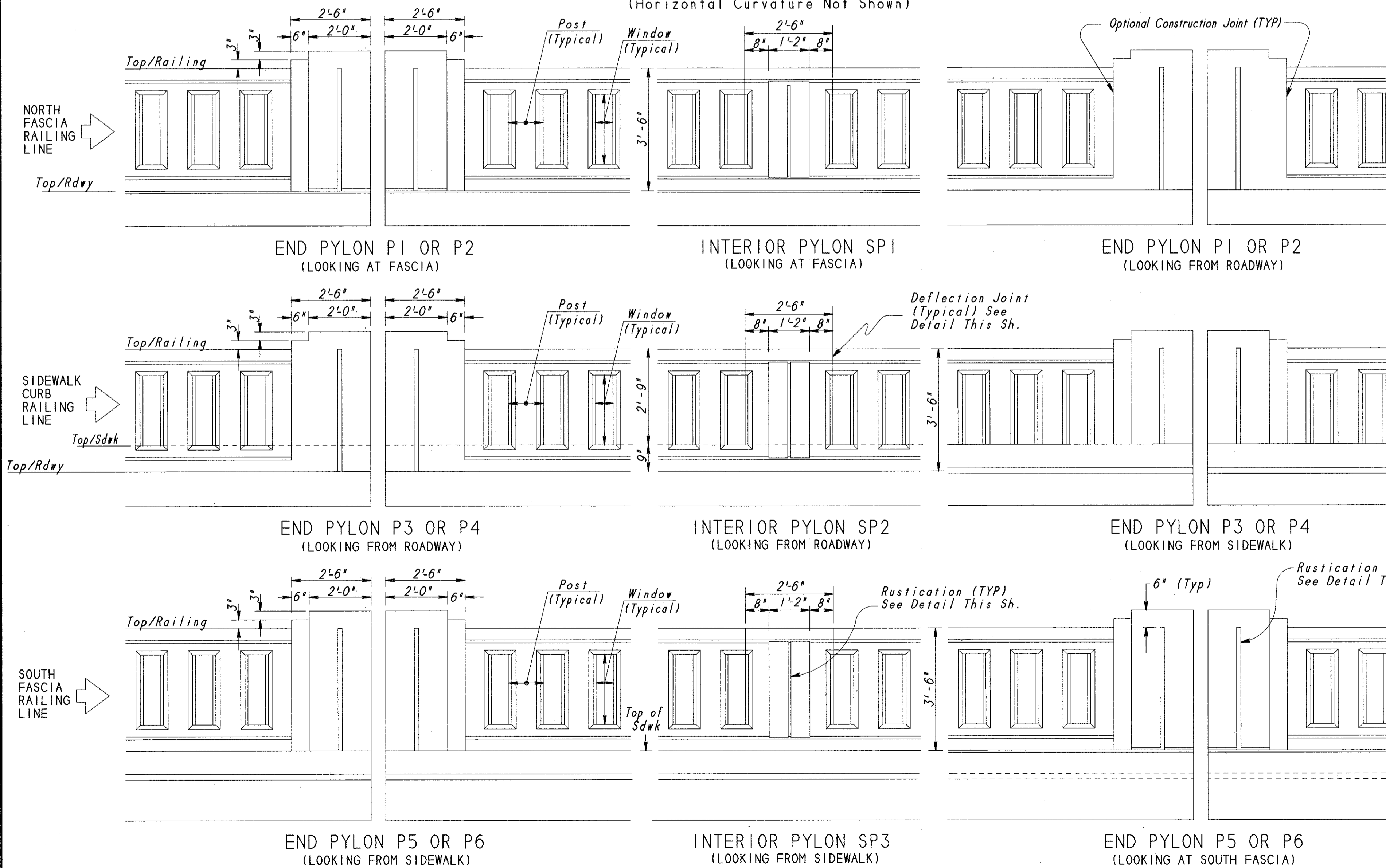
MODS/XTX Scale 8

NOTES -

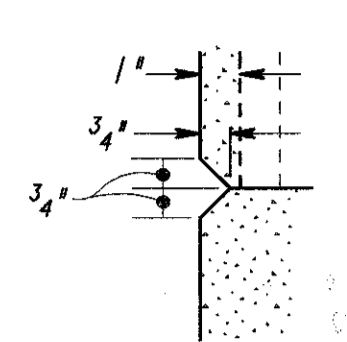
1. Work this sheet with GENERAL PLAN sheets 4/108, 5/108, 6/108, and with DECORATIVE RAILING PLAN Sheet 77/108 thru Sh. 84/108, and Sh. 85/108 With VANDAL PROTECTION FENCE
2. See GENERAL NOTES 5/5, Sh. 11/108.
3. See REINFORCING STEEL BAR LIST, Sh. 99/108
4. VANDAL PROTECTION FENCE ON RAILING and in Windows Not Shown See Sh. 85/108 For Details, Sh. 78/108 and Sh. 79/108 For Location.



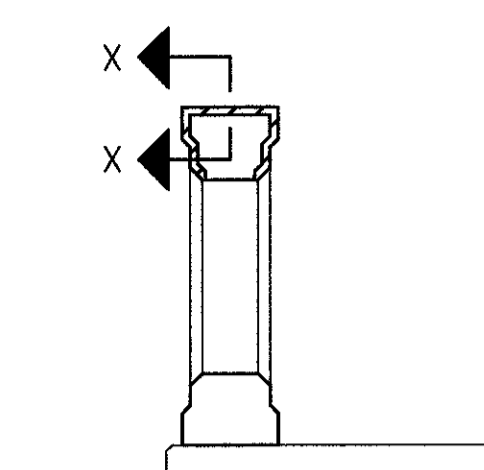
RAILING TYPICAL LONGITUDINAL SECTION VIEW
(Horizontal Curvature Not Shown)



RUSTICATION DETAIL (TYPICAL)

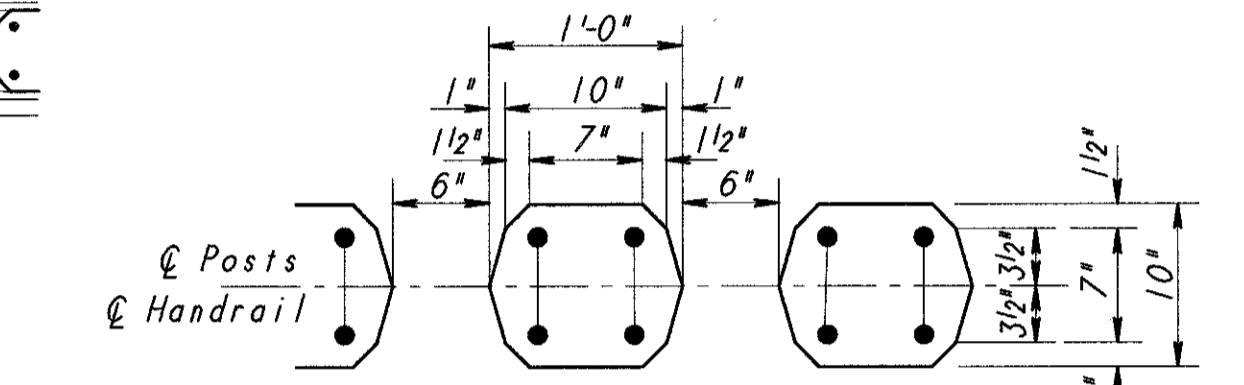


DOUBLE CHAMFER DETAIL (TYPICAL)

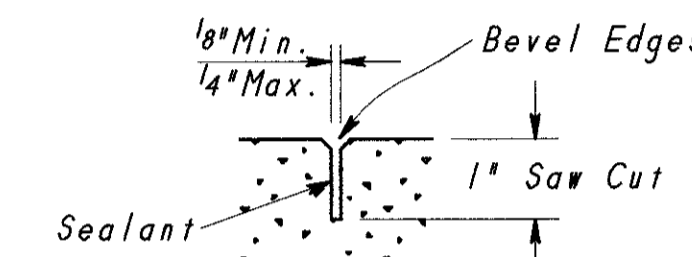


TYPICAL DEFLECTION JOINT

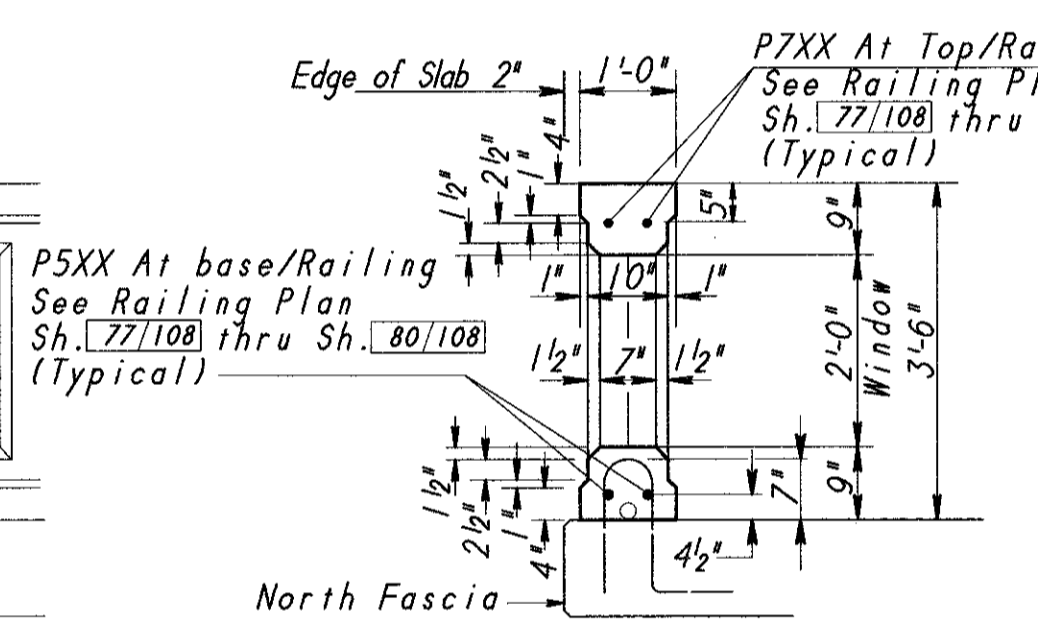
Deflection Joints Shall Be Provided By Saw Cutting Grinding and Sealing As Illustrated Above. Sealant shall be a two component gray polyurethane meeting Federal Specification TT-S-00227E, Type II Class A.



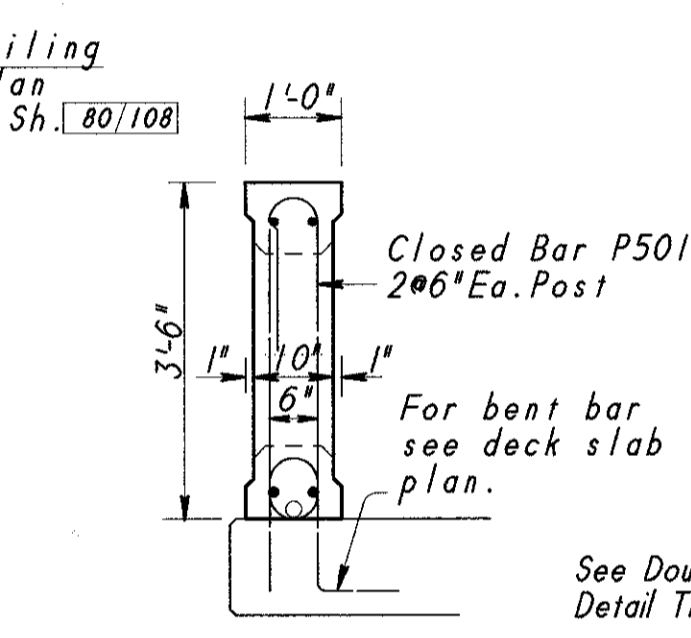
TYPICAL POST SECTION



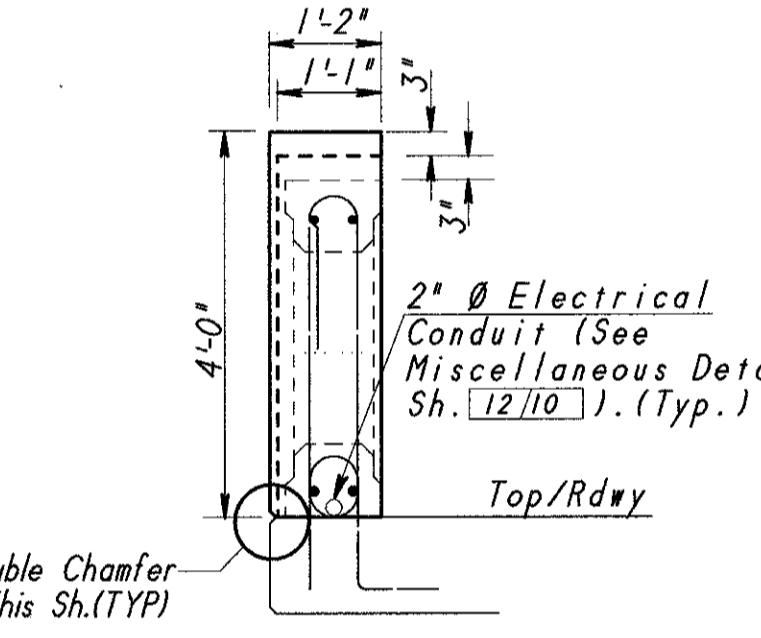
SECTION X-X



THRU WINDOW

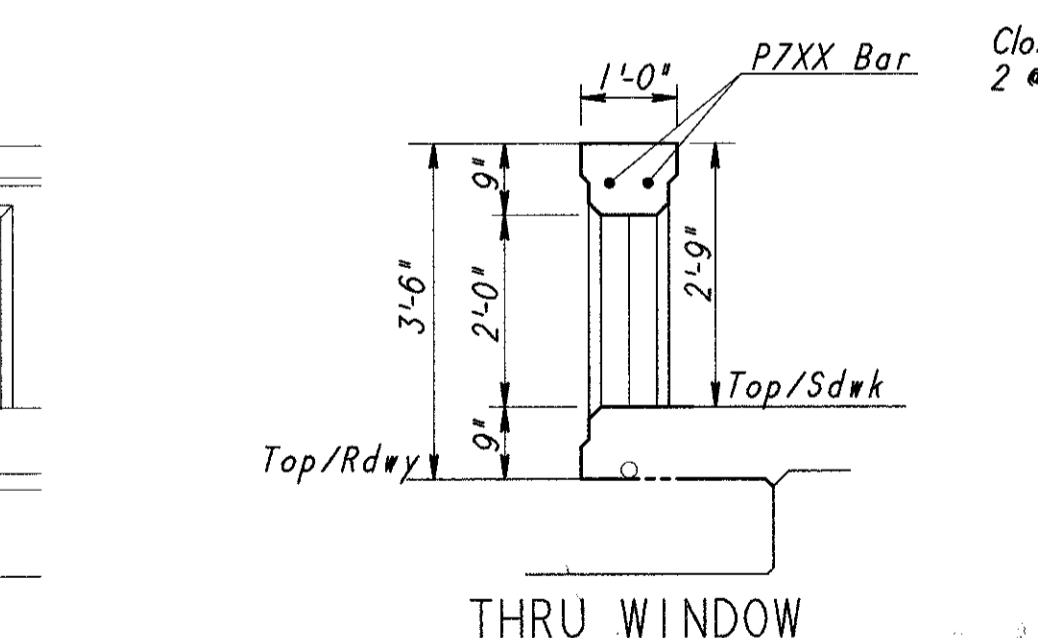


THRU POST

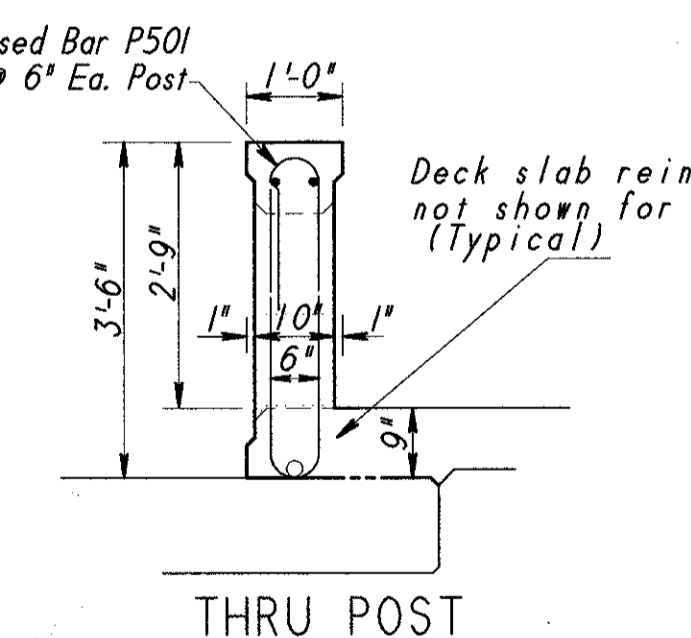


THRU END PYLON

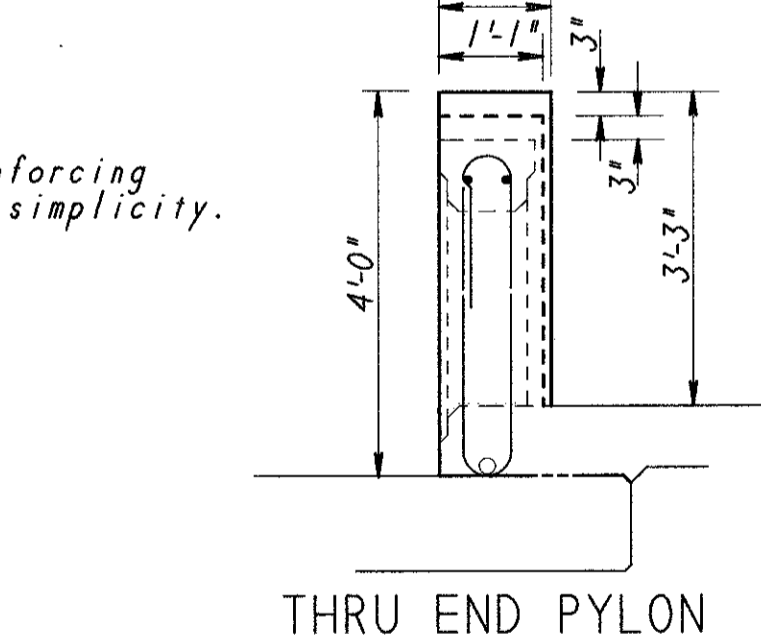
TYPICAL SECTIONS NORTH FASCIA



THRU WINDOW

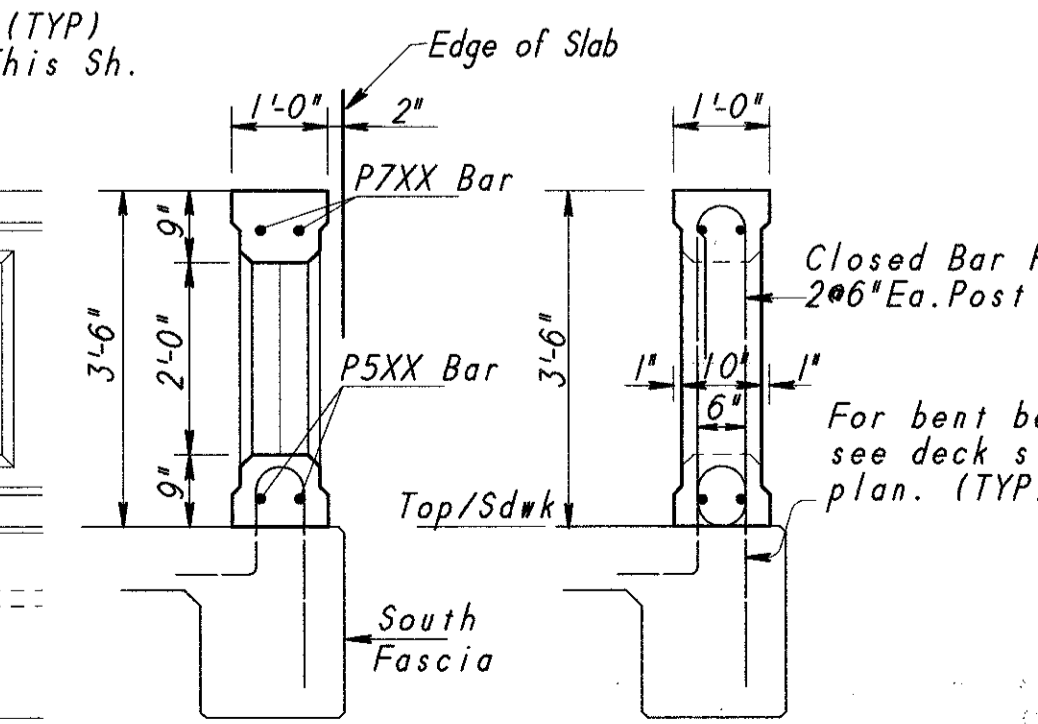


THRU POST

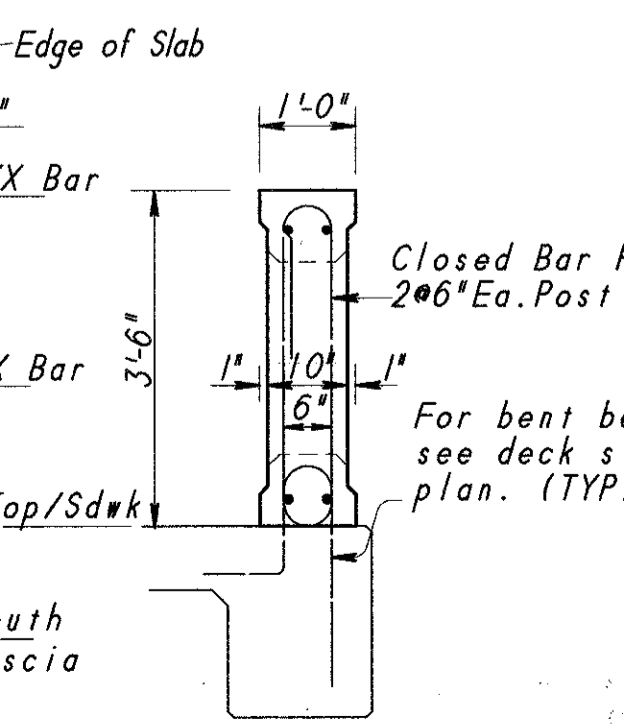


THRU END PYLON

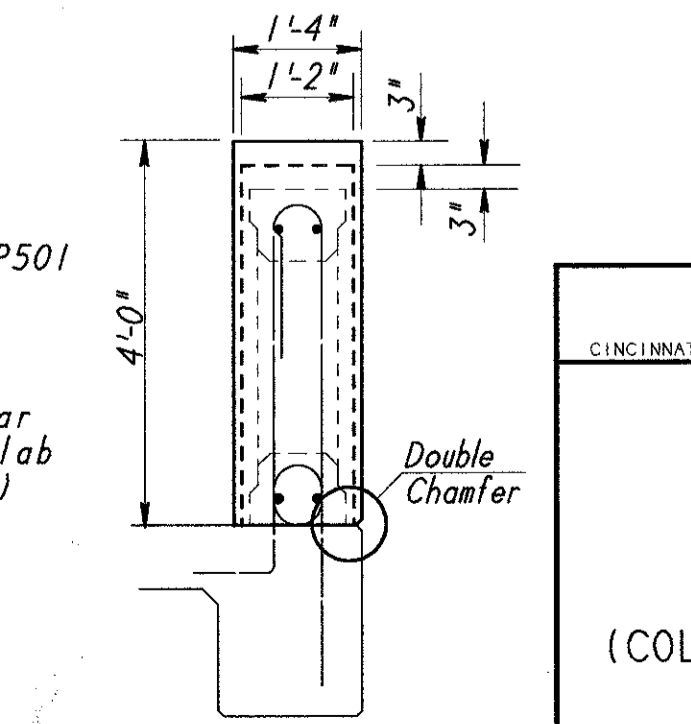
TYPICAL SECTIONS ROADWAY CURB



THRU WINDOW



THRU POST



THRU END PYLON

TYPICAL SECTIONS SOUTH FASCIA

LEGEND
P7XX Denotes No. 7 bar
P5XX " No. 5 bar

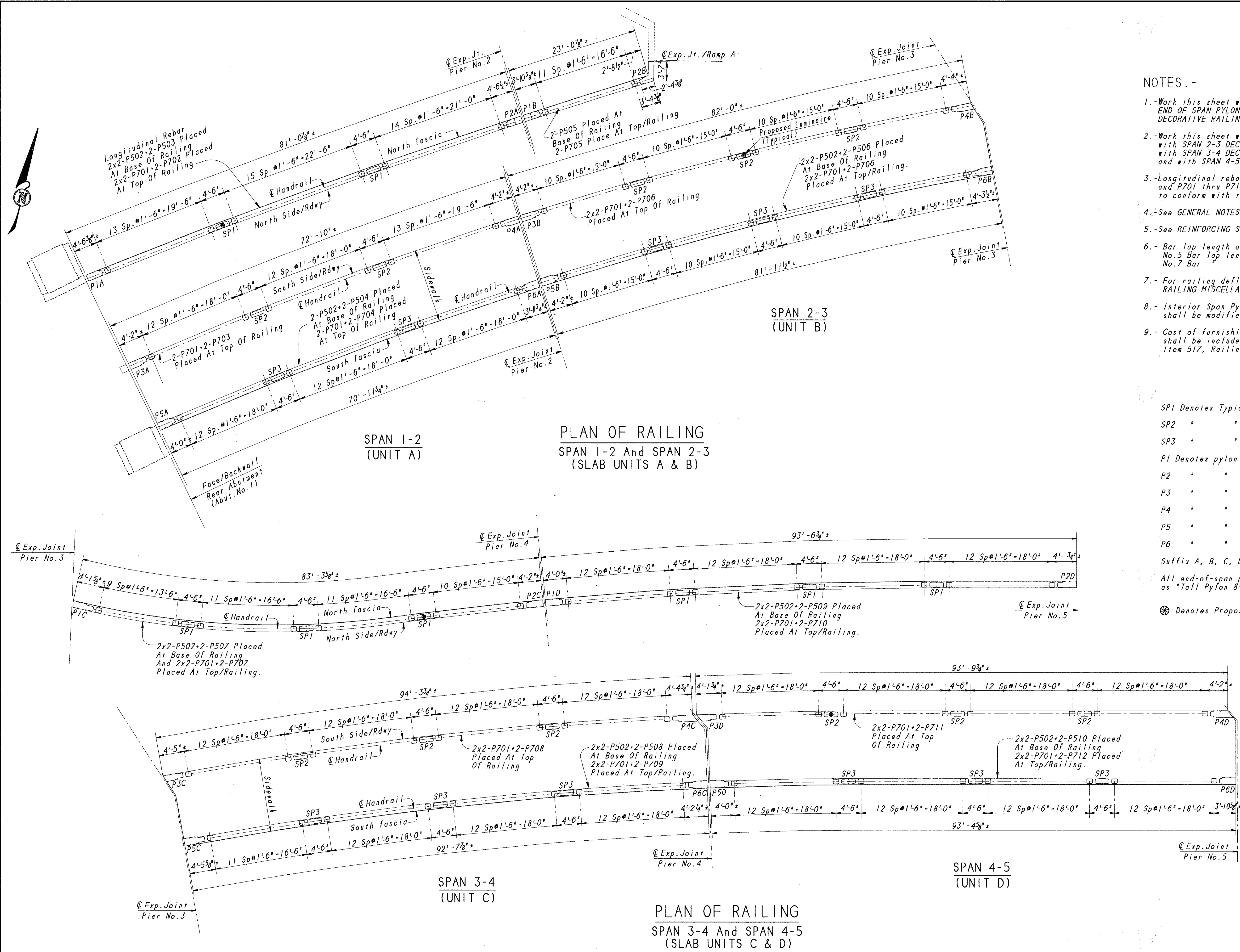
Plum, Klausmeyer & Gehrm
Consultants 76/108

DECORATIVE RAILING
MISCELLANEOUS DETAILS

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
Sta. 30+55.40
Sta. 47+16.19

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED	WDD	WDD	IEH	April 96	



NOTES -

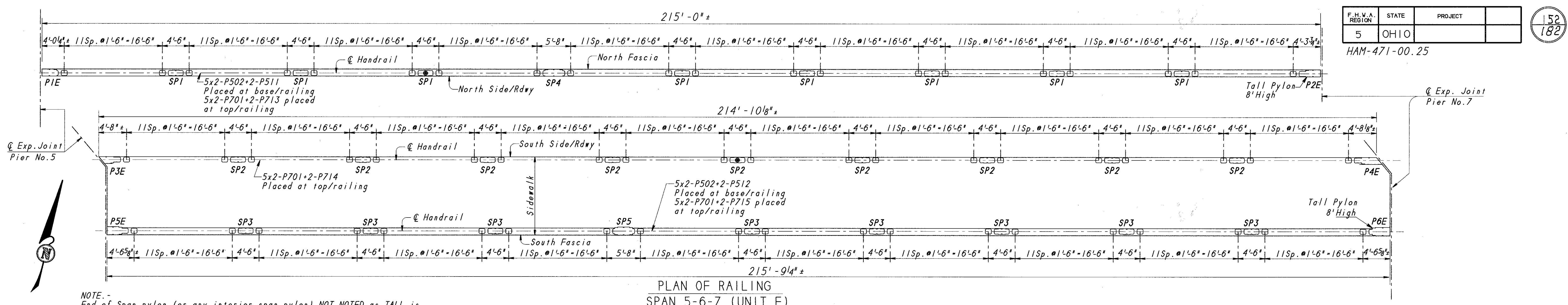
1. Work this sheet with GENERAL PLAN sheets 4/108, 5/108, 6/108, with END OF SPAN PYLONS (SPAN 1-2 THRU 4-5), Sh. 81/108 and with DECORATIVE RAILING MISCELLANEOUS DETAILS Sh. 76/108.
2. Work this sheet with SPAN 1-2 DECK SLAB UNIT A, Sh. 60/108 with SPAN 2-3 DECK SLAB UNIT B, Sh. 61/108 with SPAN 3-4 DECK SLAB UNIT C, Sh. 62/108 and with SPAN 4-5 DECK SLAB UNIT D, Sh. 63/108.
3. Longitudinal rebars P502 thru P510 along lower end of railing and P701 thru P712 along upper end, shall be bent on site to conform with the roadway alignment.
4. See GENERAL NOTES 5/5, Sh. 11/108.
5. See REINFORCING STEEL BAR LIST, Sh. 99/108.
6. Bar lap length as follows:
No. 5 Bar lap length = 24"
No. 7 Bar lap length = 42"
7. For railing deflection joints location and type, see DECORATIVE RAILING MISCELLANEOUS DETAILS, Sh. 76/108.
8. Interior Span Pylons (SP) that are to receive the proposed luminaires shall be modified as shown on MISCELLANEOUS DETAILS, Sh. 12/108.
9. Cost of furnishing and installing all reinforcing steel in railing shall be included in the Contract price bid per lin. ft. for Item 517, Railing, Concrete. (Decorative Type)

LEGEND

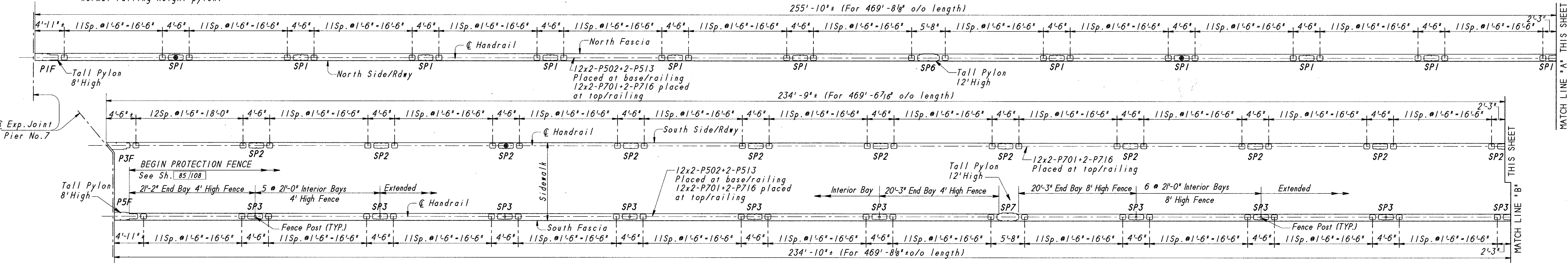
- SP1 Denotes Typical Interior Pylon at North Fascia.
 - SP2 " " " " " Curb Line.
 - SP3 " " " " " South Fascia.
 - P1 Denotes pylon at rear end of span at North Fascia.
 - P2 " " " " " forward end of span at North Fascia.
 - P3 " " " " " rear end of span at Curb side.
 - P4 " " " " " forward end of span at Curb side.
 - P5 " " " " " rear end of span at South Fascia.
 - P6 " " " " " forward end of span at South Fascia.
- Suffix A, B, C, D, etc., denotes corresponding deck slab unit.
- All end-of-span pylons are 4' high typically, except those noted as "Tall Pylon 8' High", or "Tall Pylon 12' High" on Railing Plan.
- ⊗ Denotes Proposed Luminaire

TEMPD Scale 8

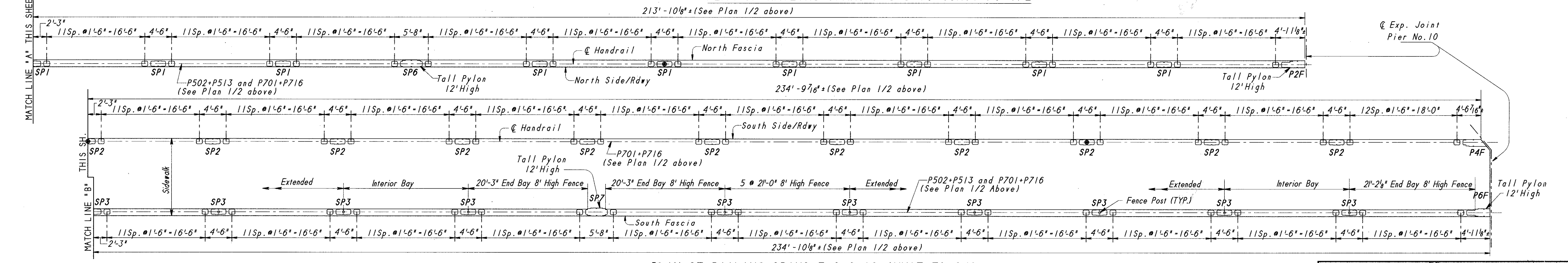
		77/108
DECORATIVE RAILING SPANS 1-2, 2-3, 3-4 & 4-5 (DECK SLAB UNITS A, B, C & D) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
Sta. 30+55.40 Sta. 47+16.19		
HAMILTON COUNTY		
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96
REVISIONS		



NOTE: -
End of Span pylon (or any interior span pylon) NOT NOTED as TALL is normal railing height pylon.



PLAN OF RAILING SPANS 7-8-9-10 (UNIT F) 1/2



PLAN OF RAILING SPANS 7-8-9-10 (UNIT F) 2/2

- NOTES: -
1. - Work this sheet with DECORATIVE RAILING END OF SPAN PYLONS (Span 5-6-7 and 7-8-9-10), Sh. 82/108 And With Decorative Railing Miscellaneous Details Sh. 76/108.
 2. - Work this sheet with GENERAL PLAN 1/3 Sh. 4/108 and with GENERAL PLAN 2/3 Sh. 5/108 with SPAN 5-6-7, DECK SLAB UNIT E, Sh. 64/108 with SPAN 7-8-9-10 DECK SLAB UNIT F 1/2 Sh. 65/108 and with SPAN 7-8-9-10 DECK SLAB UNIT F 2/2 Sh. 66/108.
 3. - See GENERAL NOTES 5/5 Sh. 11/108.
 4. - See REINFORCING STEEL LIST on Sh. 99/108.
 5. - For additional Notes and Legend See Sh. 77/108.

Pflum, Klausmeyer & Gehrm
CINCINNATI, OHIO
Consultants

DECORATIVE RAILING
SPANS 5-6-7 & 7-8-9-10
(UNIT E & UNIT F)
BRIDGE No. HAM471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
DESIGNED: ED
DRAWN: ED
TRACED: WDD
CHECKED: WDD
REVIEWED: IEH April 96
DATE: 96
REVISION: 1

Sta. 30+55.40
Sta. 47+16.19

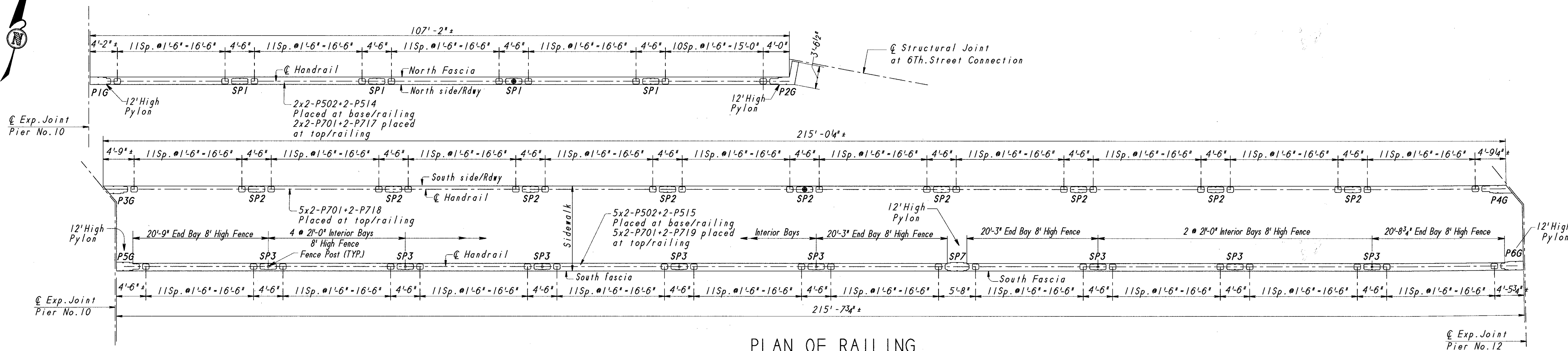
TEKEE Scale 8

NOTE -
End of Span pylon (or any interior span pylon) NOT NOTED as TALL is normal railing height pylon.

F.H.W.A. REGION	STATE	PROJECT
5	OHIO	

153
182

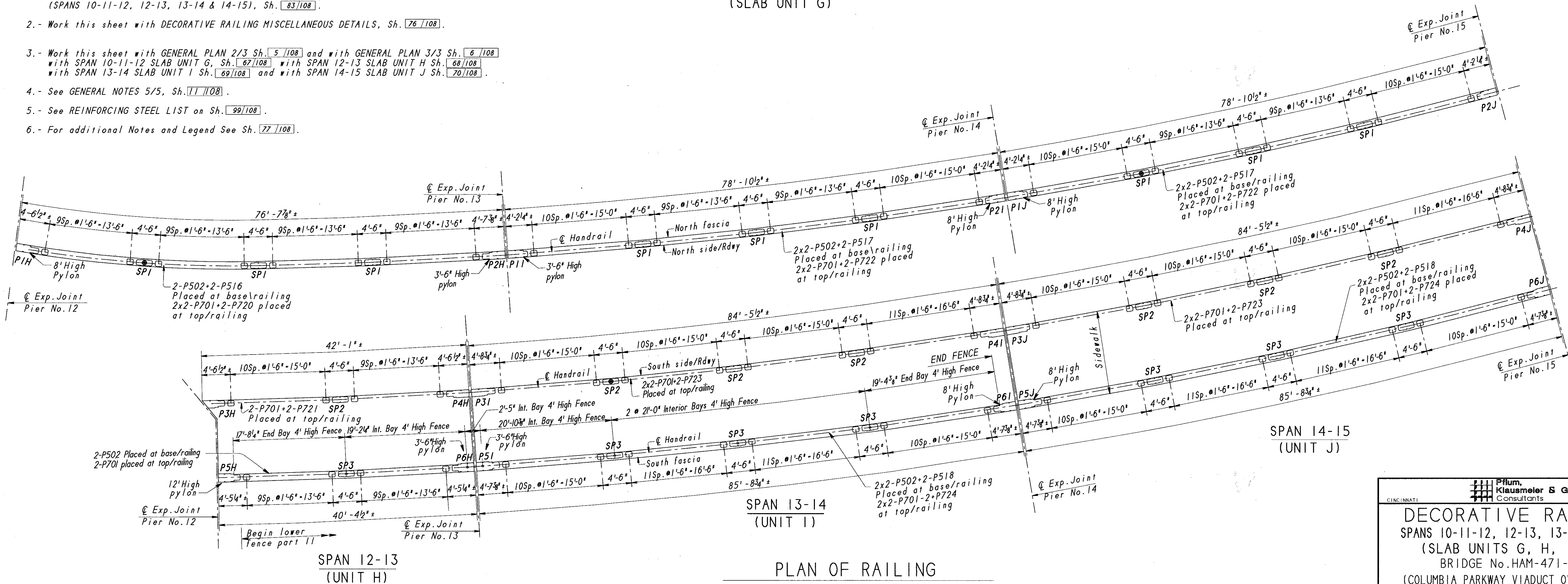
HAM-471-00.25



PLAN OF RAILING
SPAN 10-11-12
(SLAB UNIT G)

NOTES -

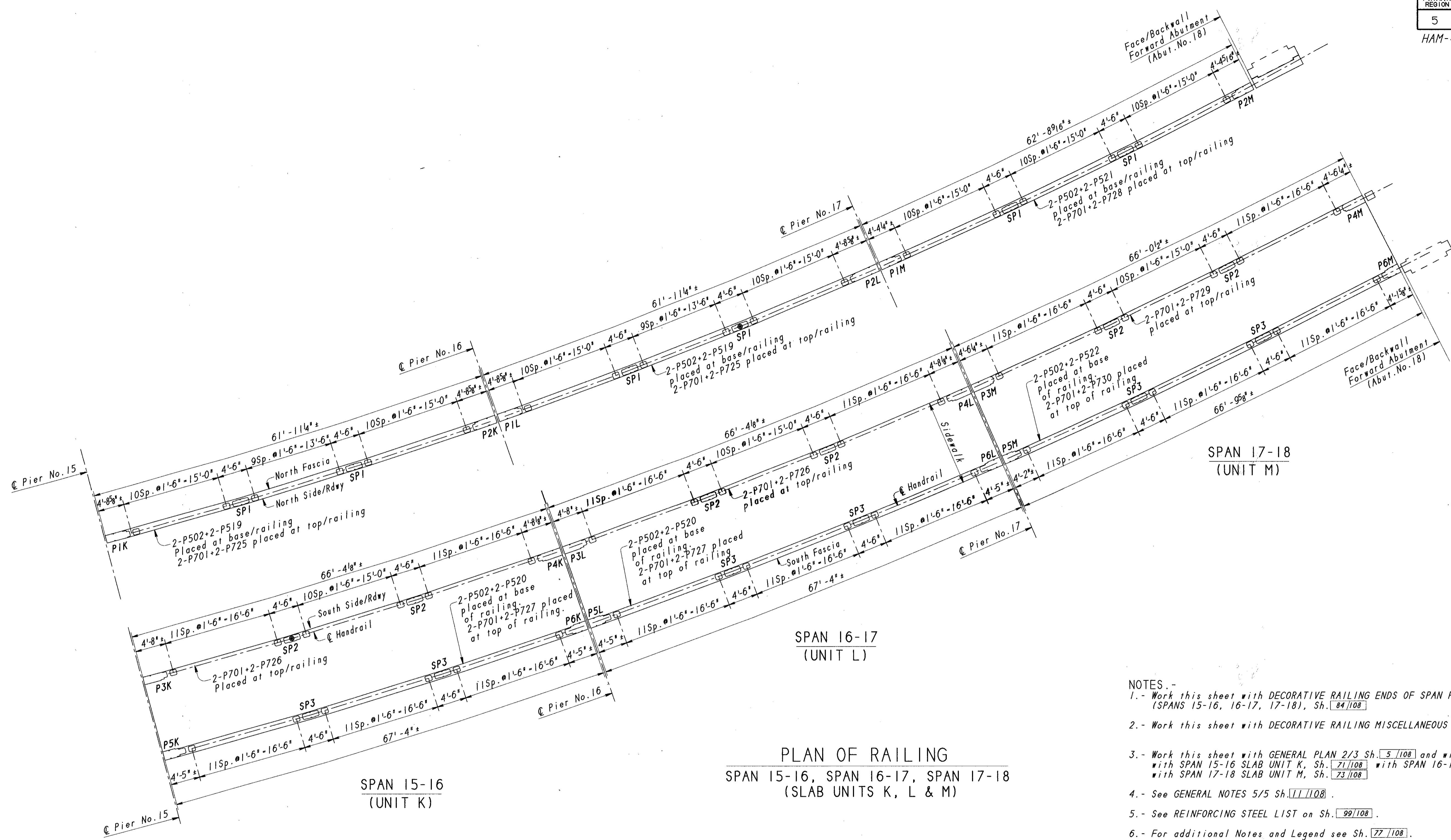
1. - Work this sheet with DECORATIVE RAILING END OF SPAN PYLONS (SPANS 10-11-12, 12-13, 13-14 & 14-15), Sh. [83/108].
2. - Work this sheet with DECORATIVE RAILING MISCELLANEOUS DETAILS, Sh. [76/108].
3. - Work this sheet with GENERAL PLAN 2/3 Sh. [5/108] and with GENERAL PLAN 3/3 Sh. [6/108] with SPAN 10-11-12 SLAB UNIT G, Sh. [67/108] with SPAN 12-13 SLAB UNIT H Sh. [68/108] with SPAN 13-14 SLAB UNIT I Sh. [69/108] and with SPAN 14-15 SLAB UNIT J Sh. [70/108].
4. - See GENERAL NOTES 5/5, Sh. [11/108].
5. - See REINFORCING STEEL LIST on Sh. [99/108].
6. - For additional Notes and Legend See Sh. [77/108].



PLAN OF RAILING
SPAN 12-13, SPAN 13-14, SPAN 14-15
(SLAB UNITS H, I & J)

CINCINNATI		Plum, Klausmeier & Gehrum Consultants		79/108
DECORATIVE RAILING				
SPANS 10-11-12, 12-13, 13-14 & 14-15 (SLAB UNITS G, H, I & J) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY Sls. 30-55.40 Sls. 47-16.19				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
ED	ED		WDD	IEH April 96
				REVISED

TEKH/J Scale 8



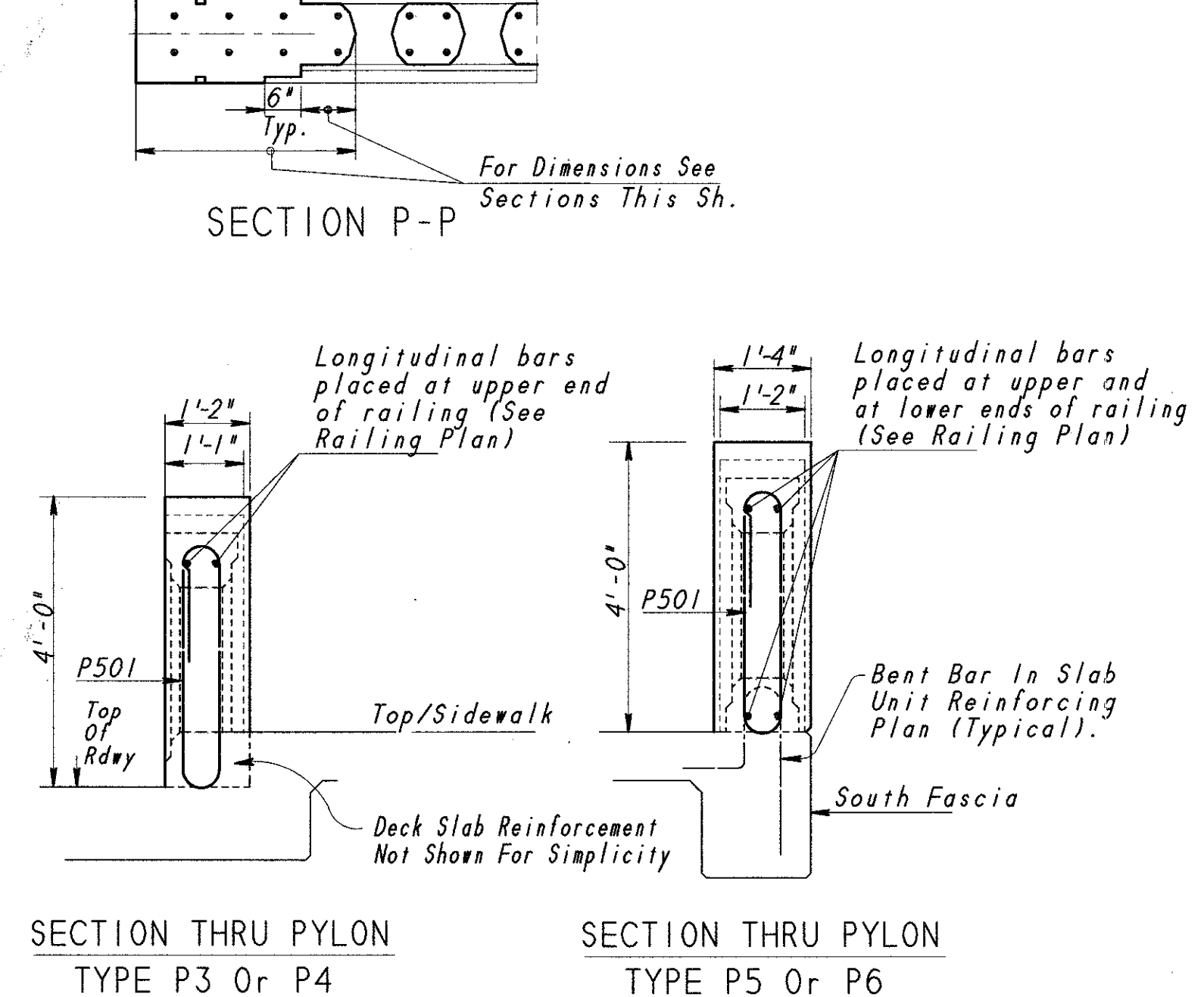
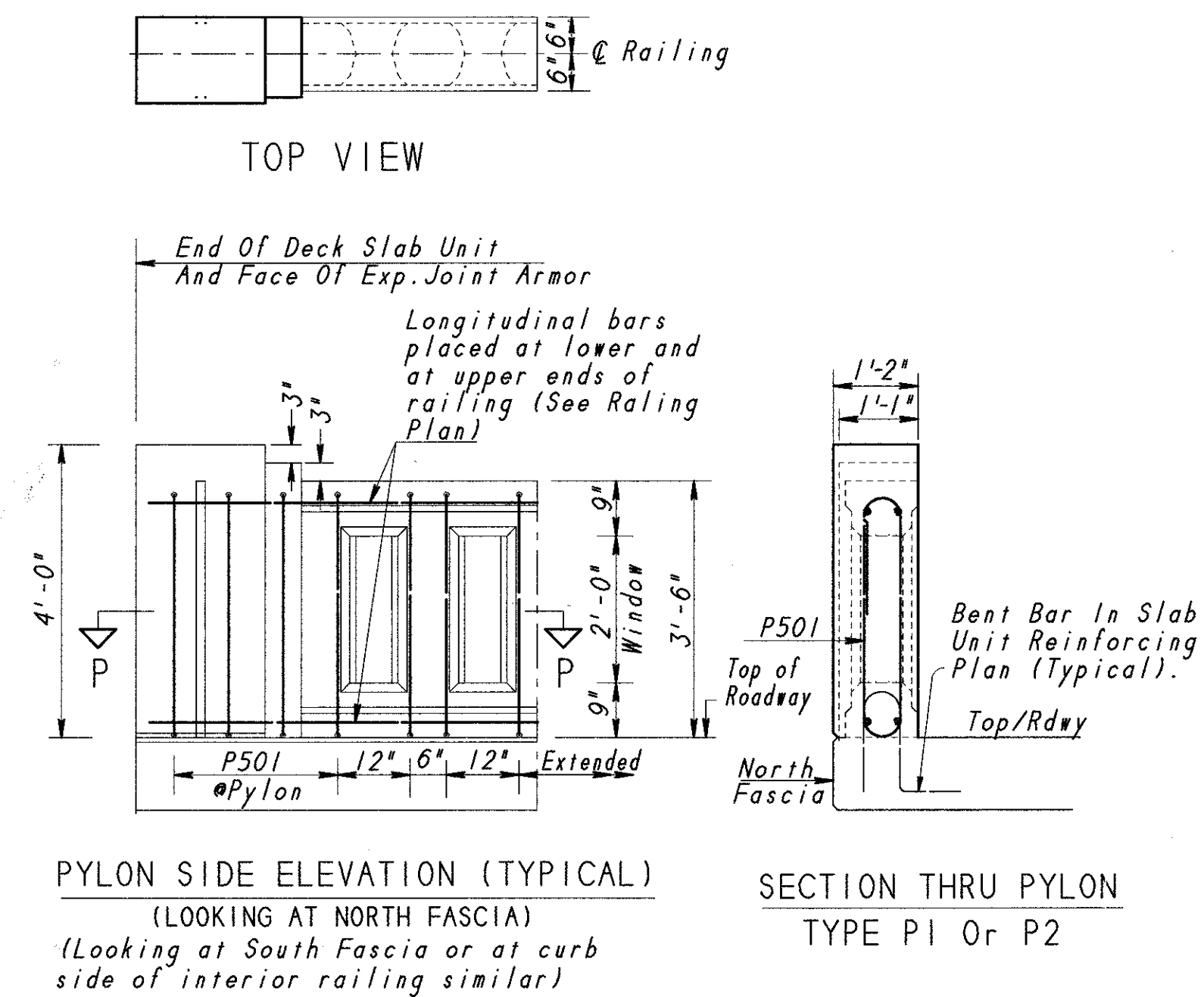
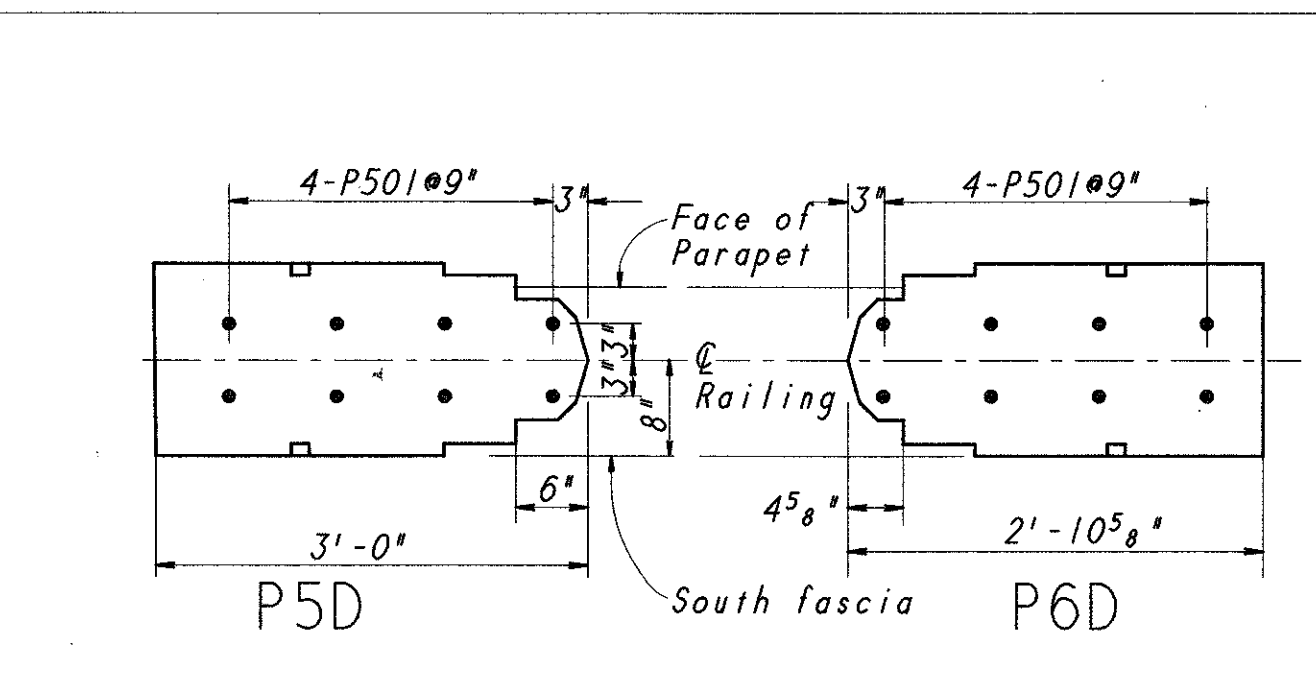
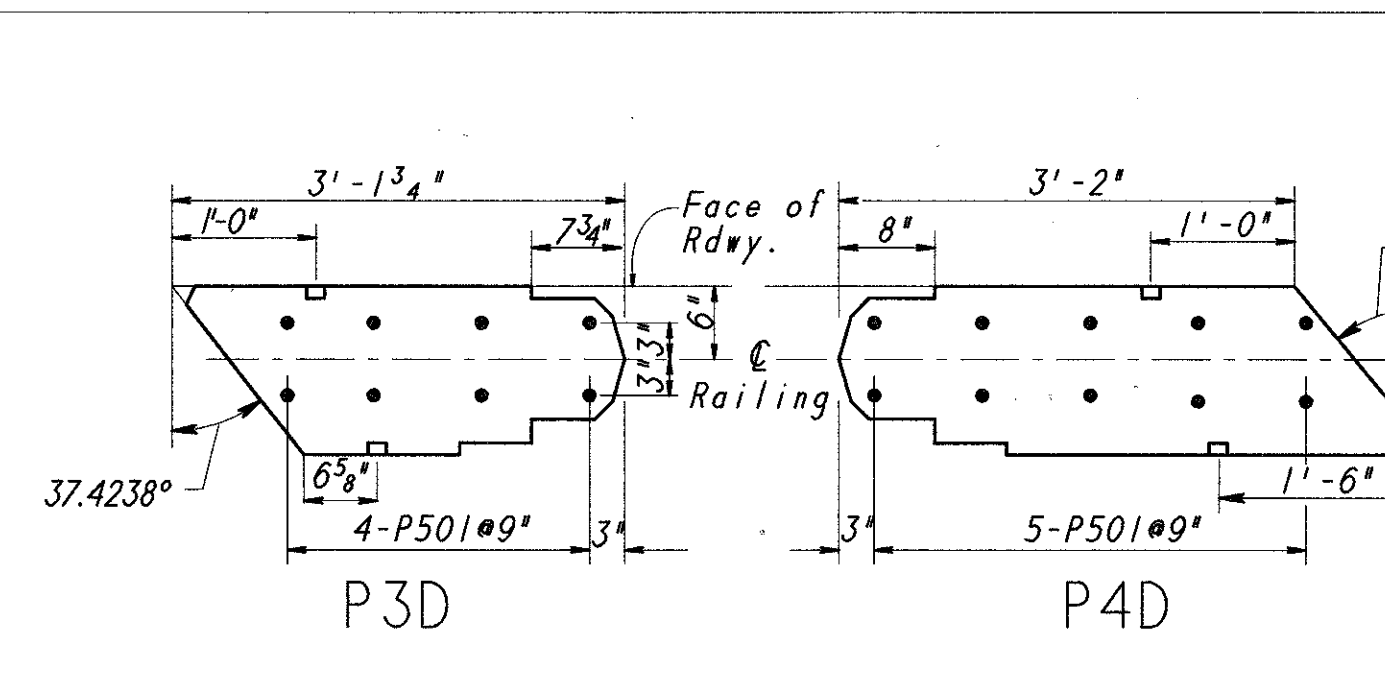
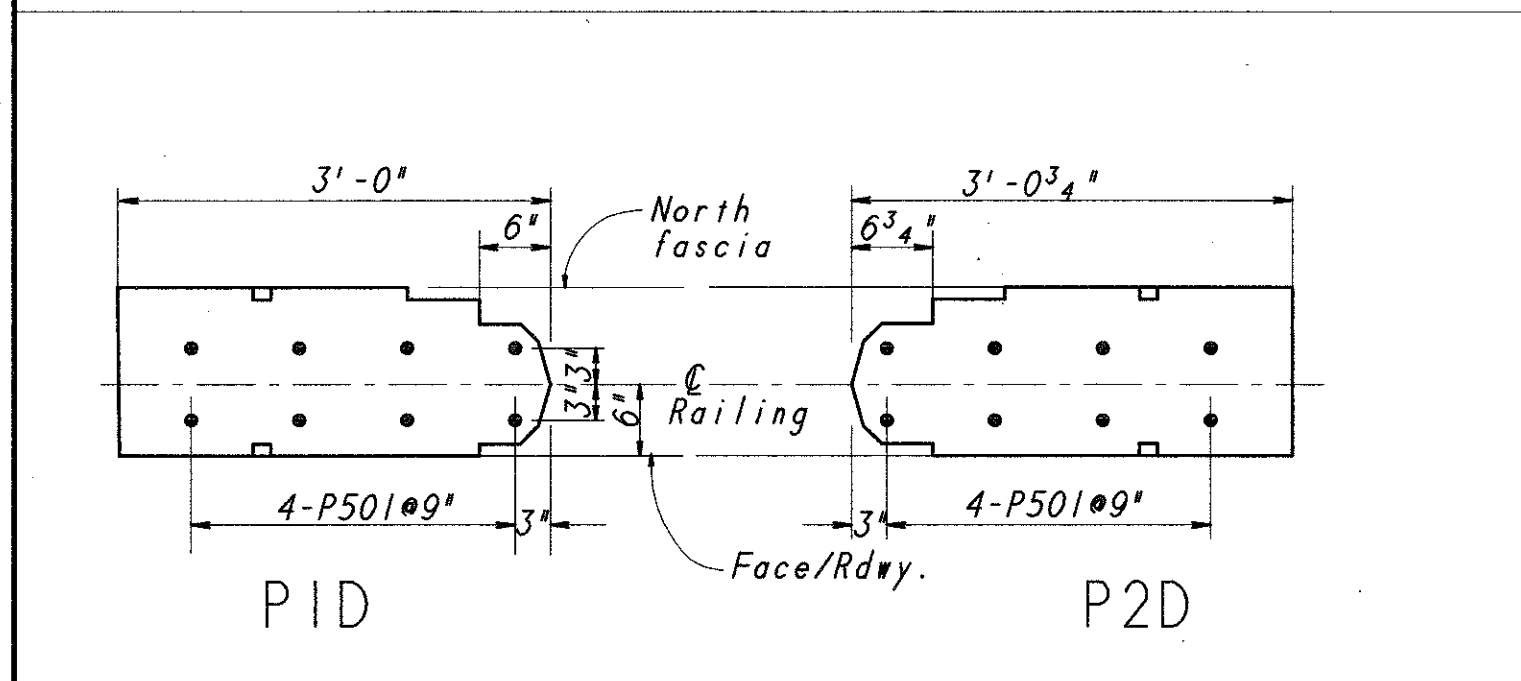
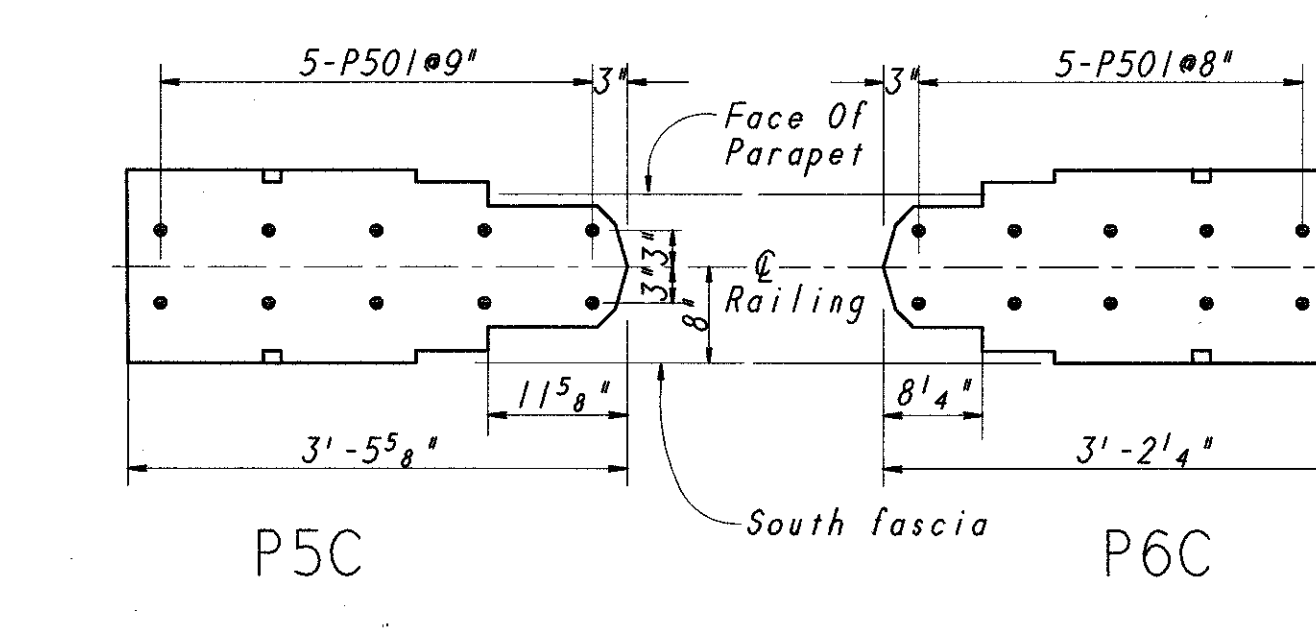
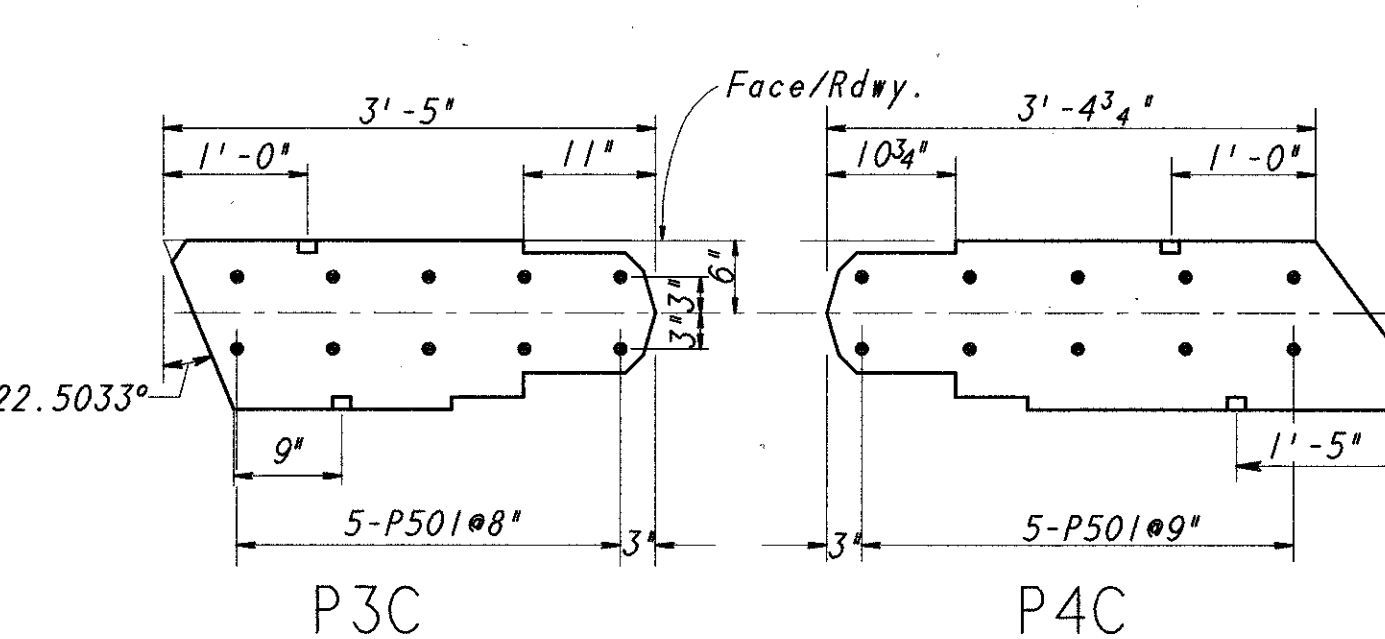
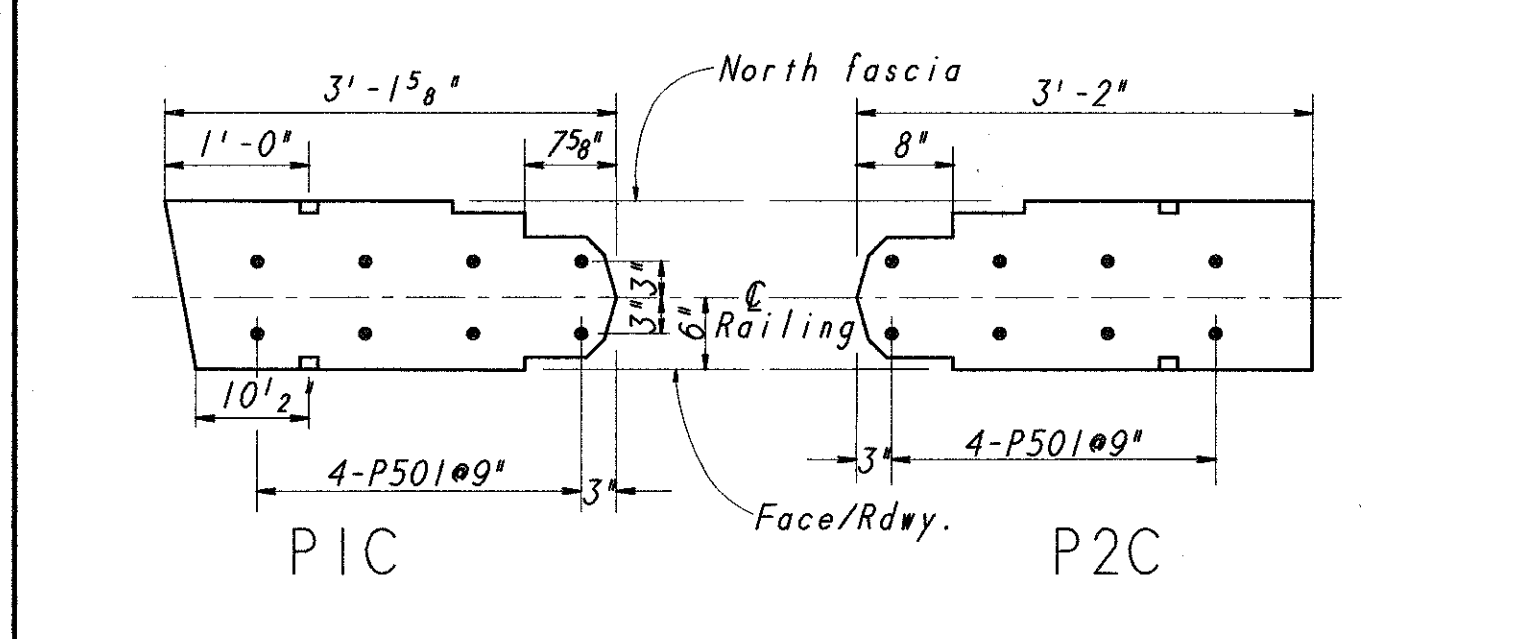
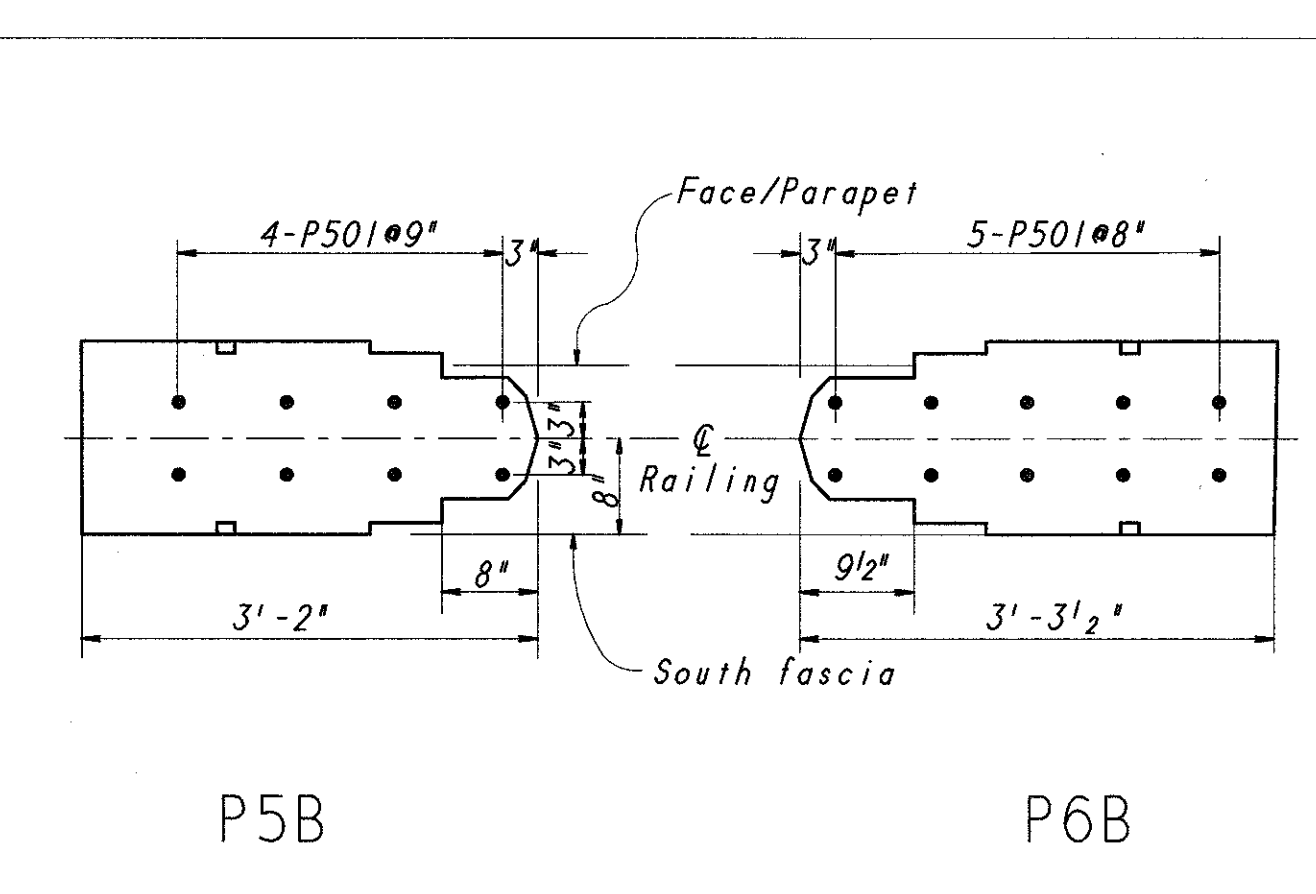
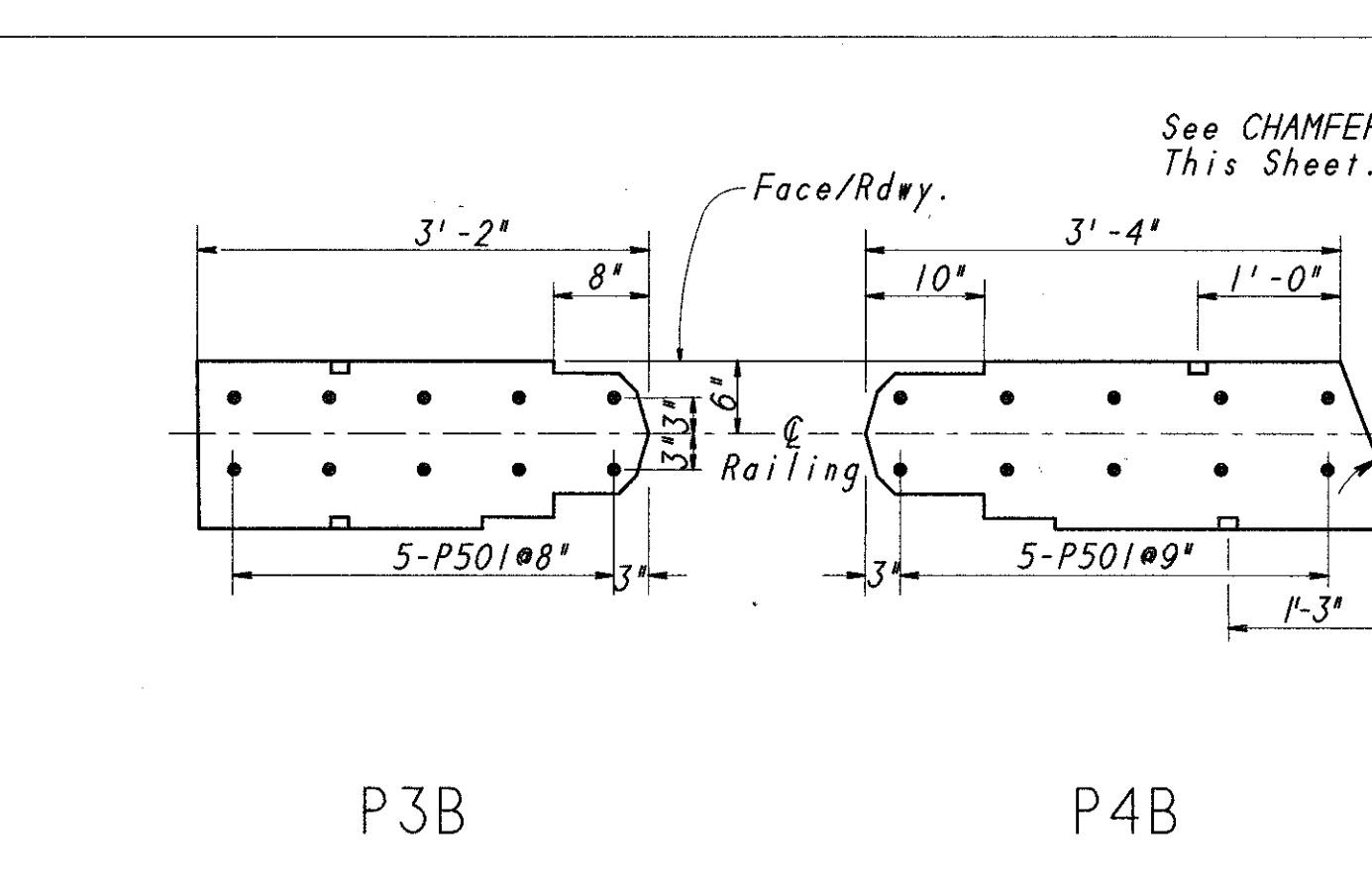
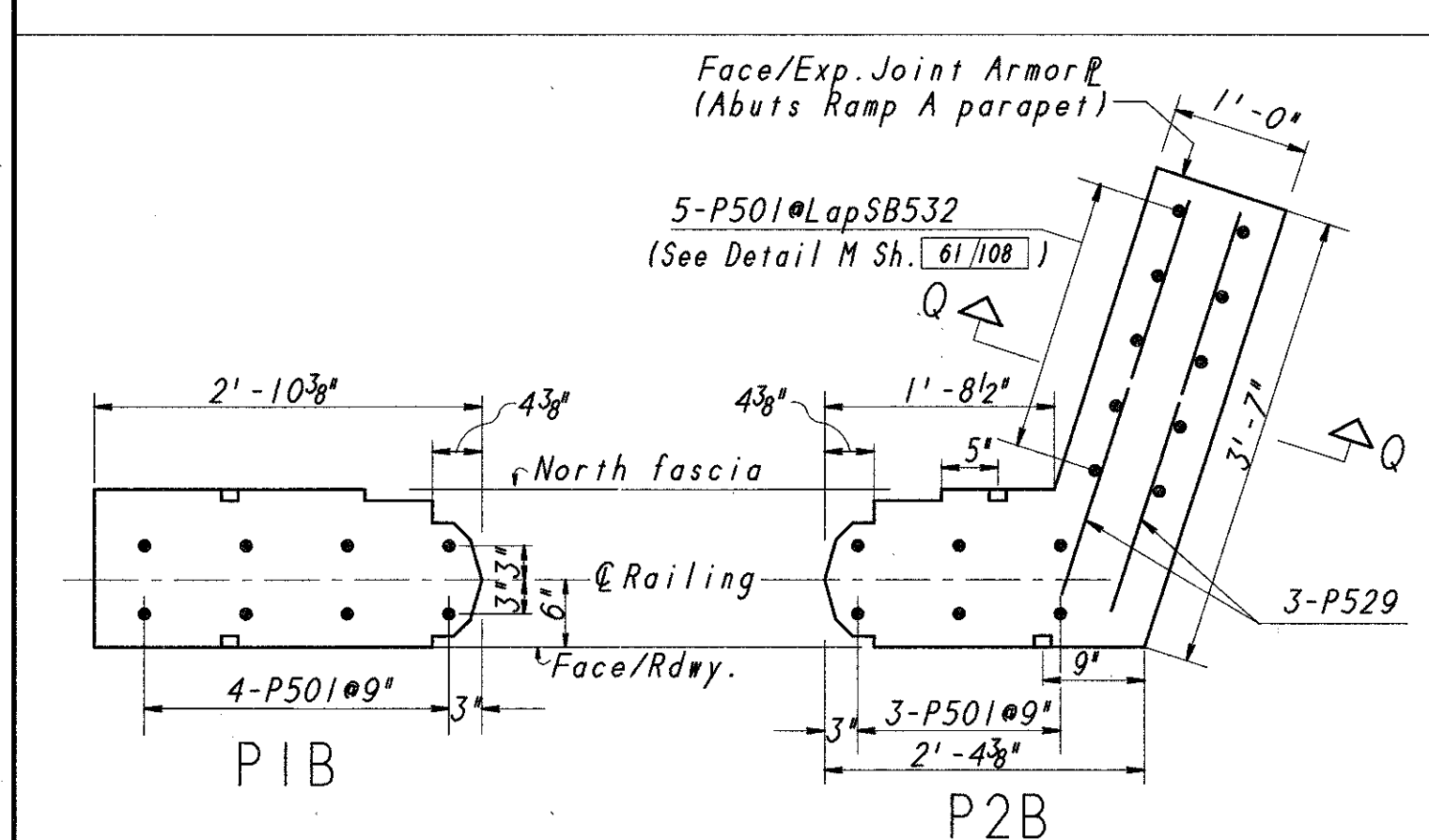
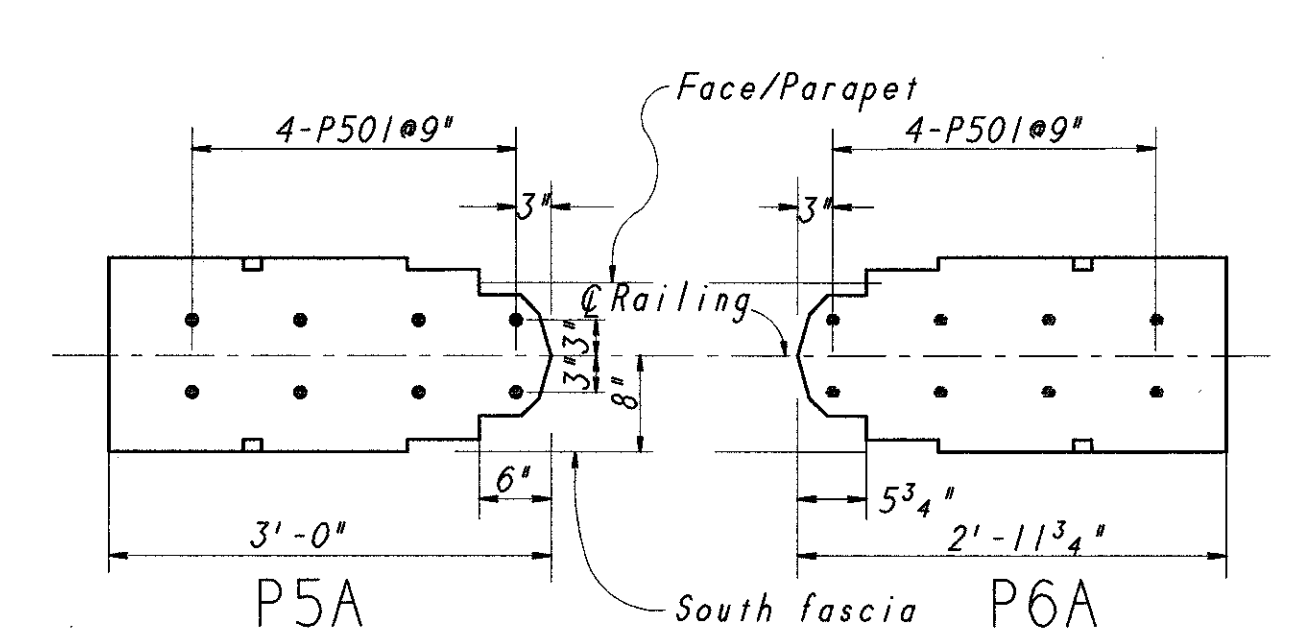
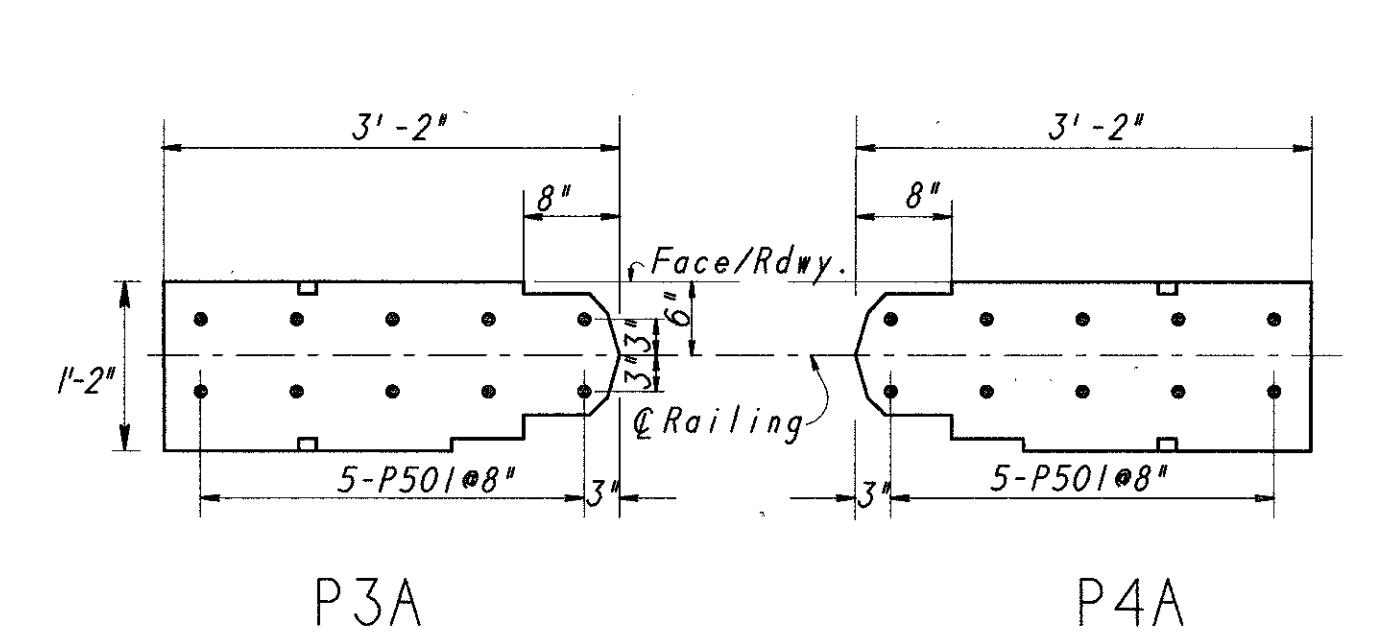
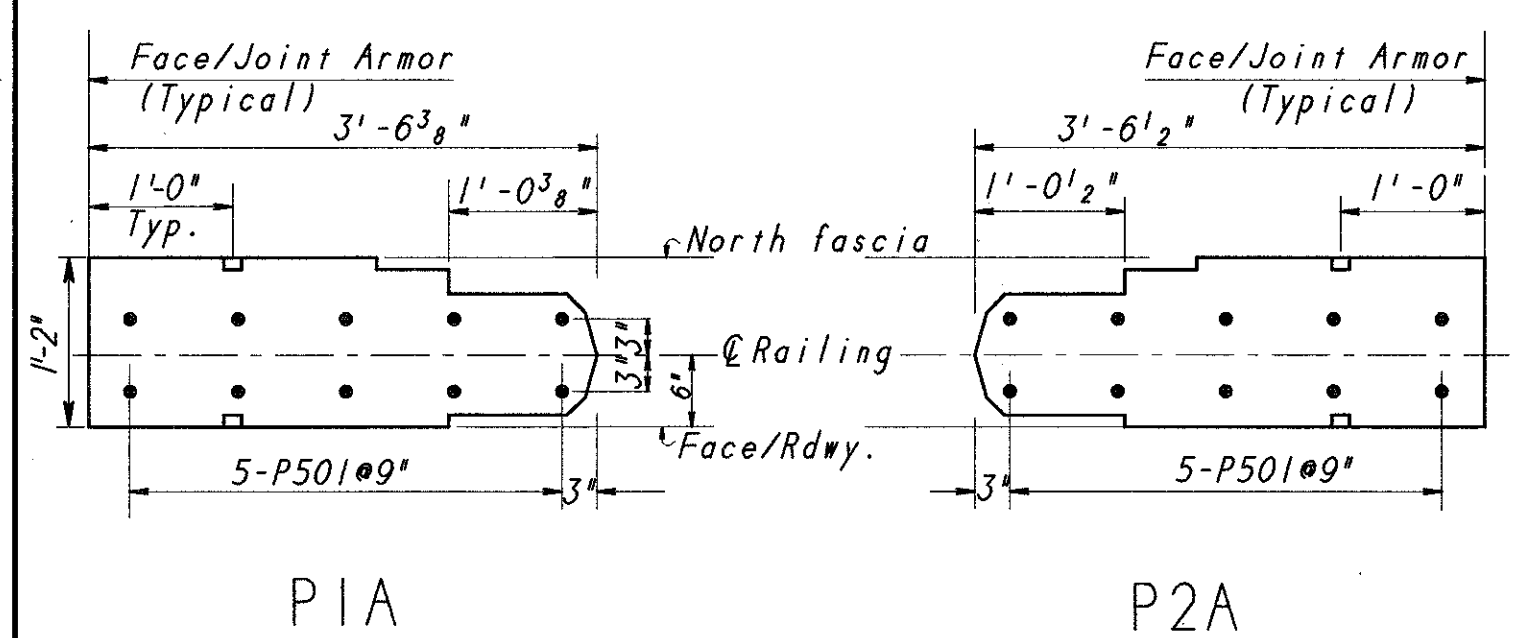
SPAN 17-18
(UNIT M)

SPAN 16-17
(UNIT L)

PLAN OF RAILING
SPAN 15-16, SPAN 16-17, SPAN 17-18
(SLAB UNITS K, L & M)

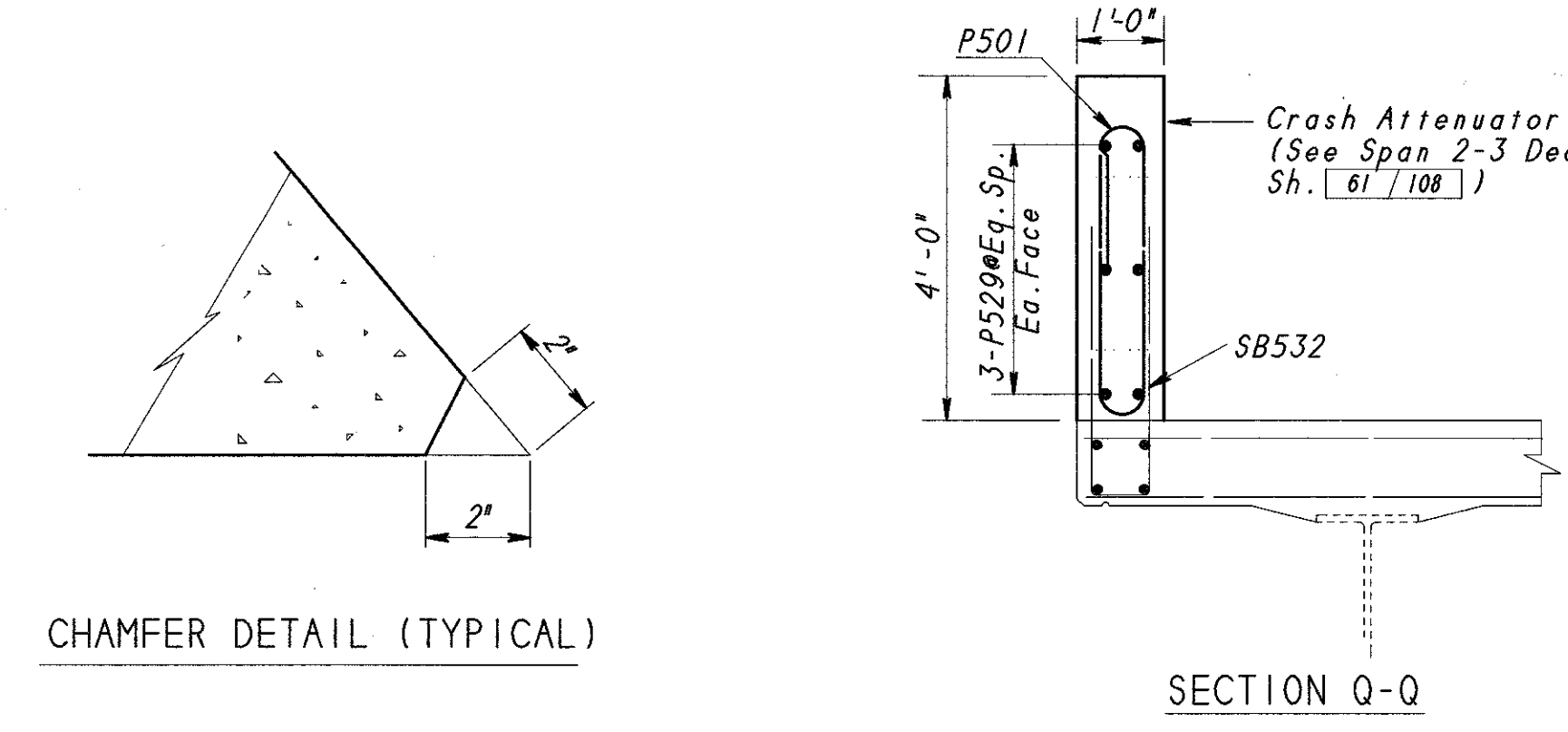
- NOTES -
1. - Work this sheet with DECORATIVE RAILING ENDS OF SPAN PYLONS SCHEDULE (SPANS 15-16, 16-17, 17-18), Sh. 84/108.
 2. - Work this sheet with DECORATIVE RAILING MISCELLANEOUS DETAILS, Sh. 76/108.
 3. - Work this sheet with GENERAL PLAN 2/3 Sh. 5/108 and with GENERAL PLAN 3/3 Sh. 6/108 with SPAN 15-16 SLAB UNIT K, Sh. 71/108 with SPAN 16-17 SLAB UNIT L, Sh. 72/108 with SPAN 17-18 SLAB UNIT M, Sh. 73/108.
 4. - See GENERAL NOTES 5/5 Sh. 11/108.
 5. - See REINFORCING STEEL LIST on Sh. 99/108.
 6. - For additional Notes and Legend see Sh. 77/108.

Pflum, Klausmeier & Gehrum CONSULTANTS		80/108
DECORATIVE RAILING SPANS 15-16, 16-17 & 17-18 (SLAB UNITS K, L & M) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	TRACED WDD
CHECKED WDD	REVIEWED IEH April 96	DATE REVISID



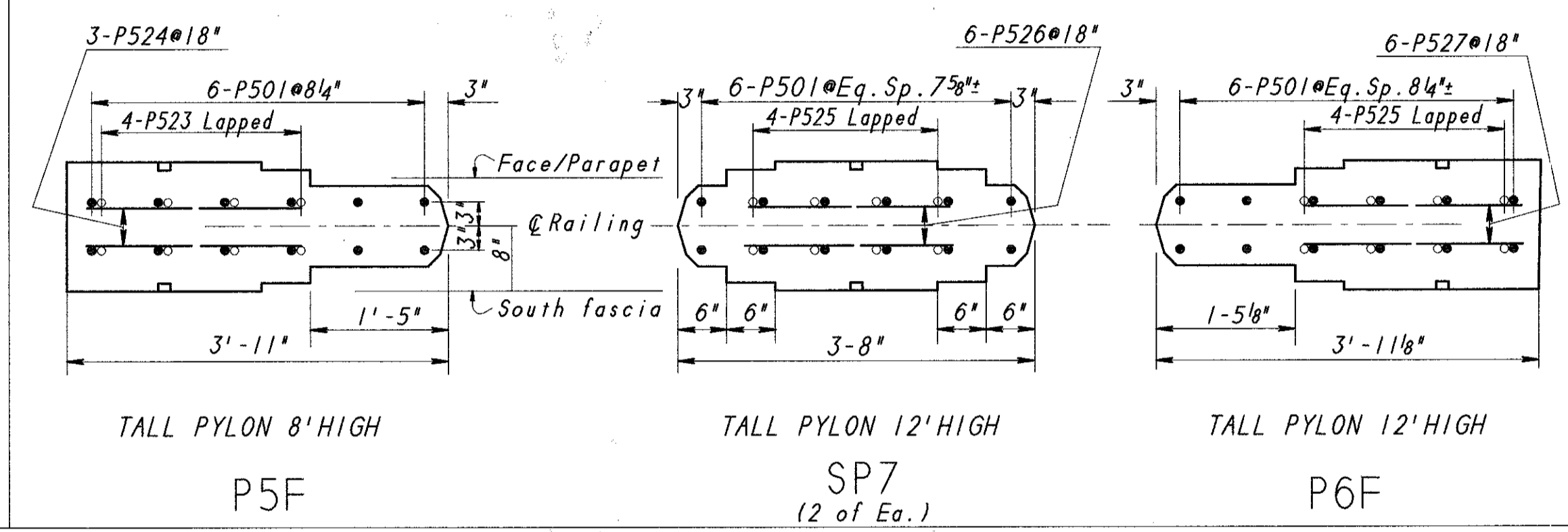
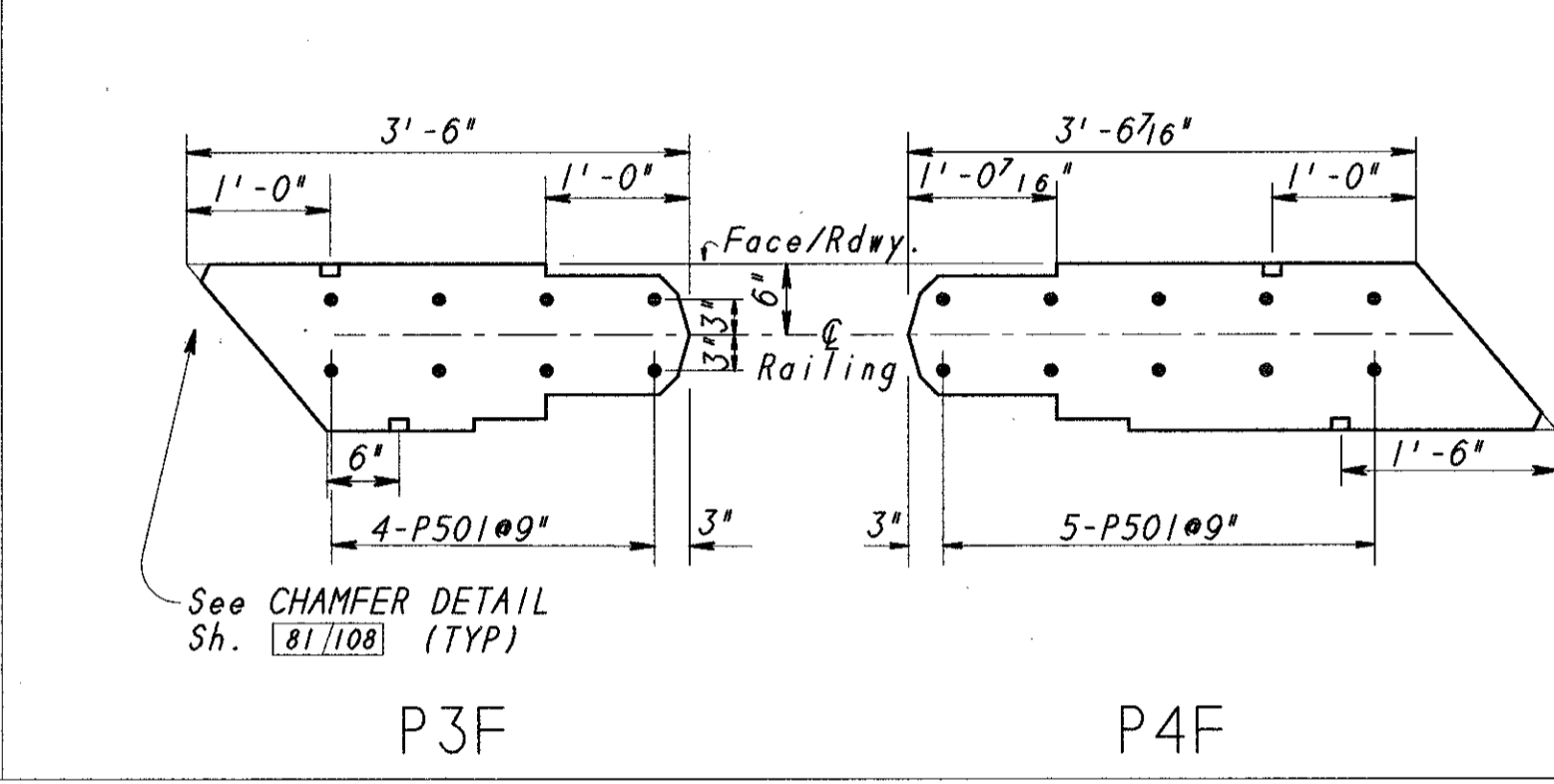
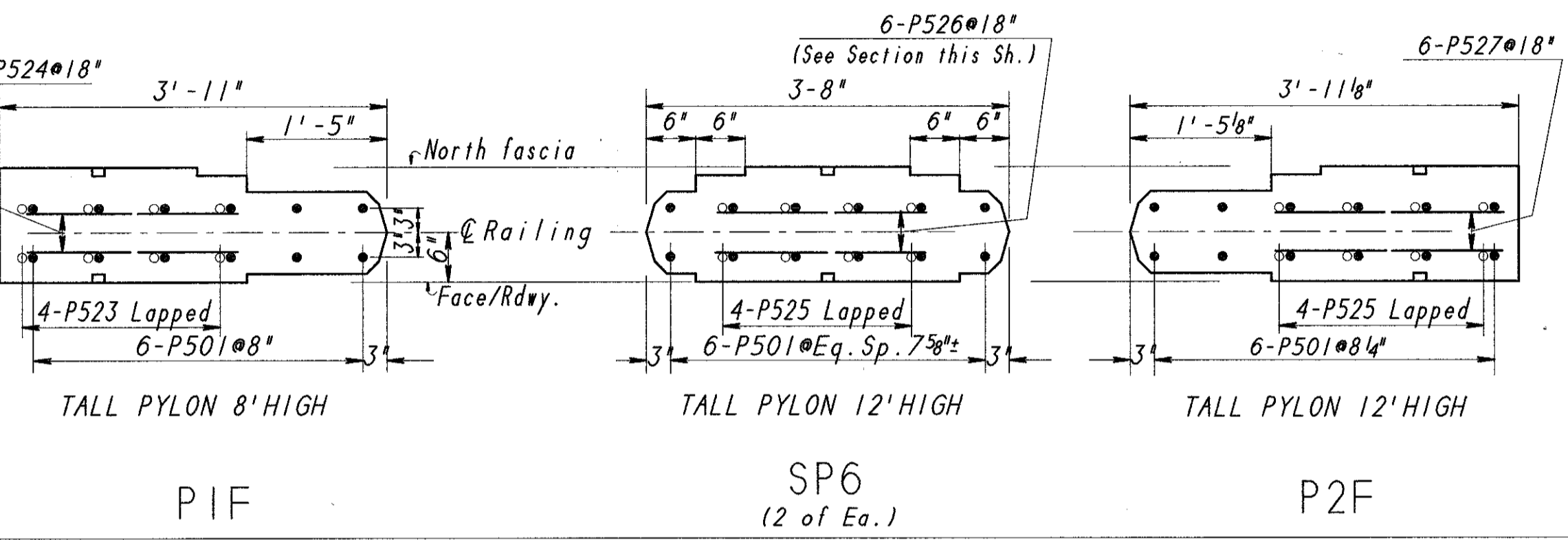
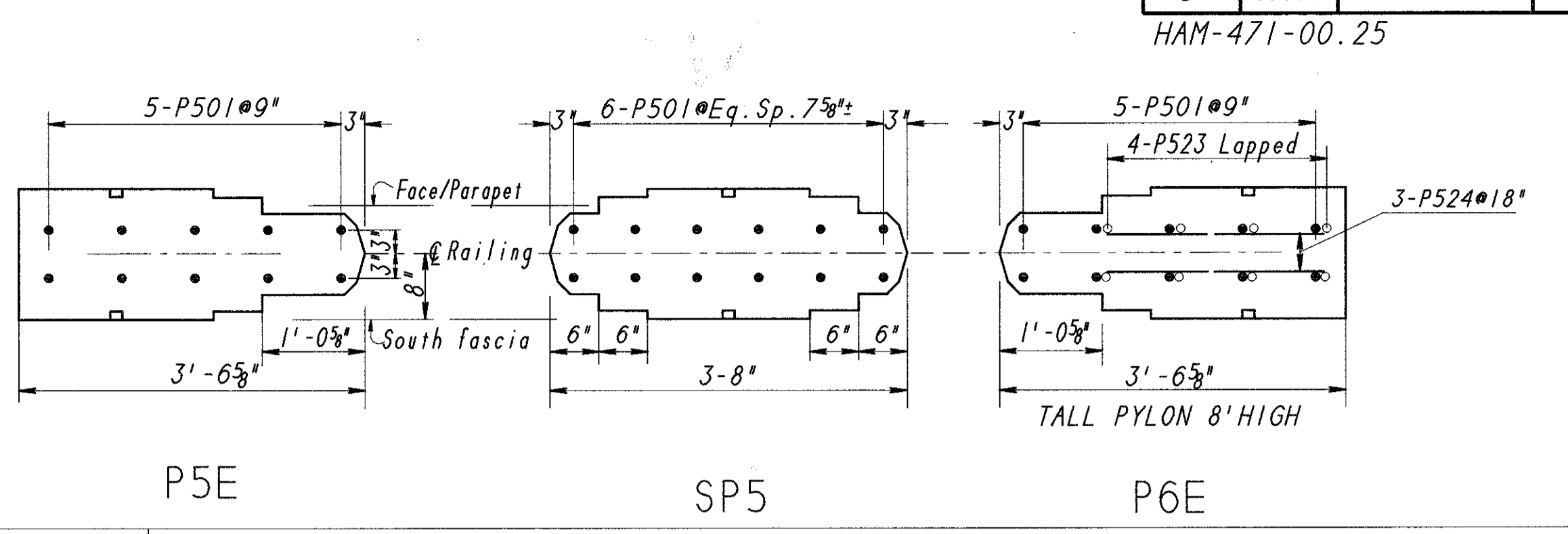
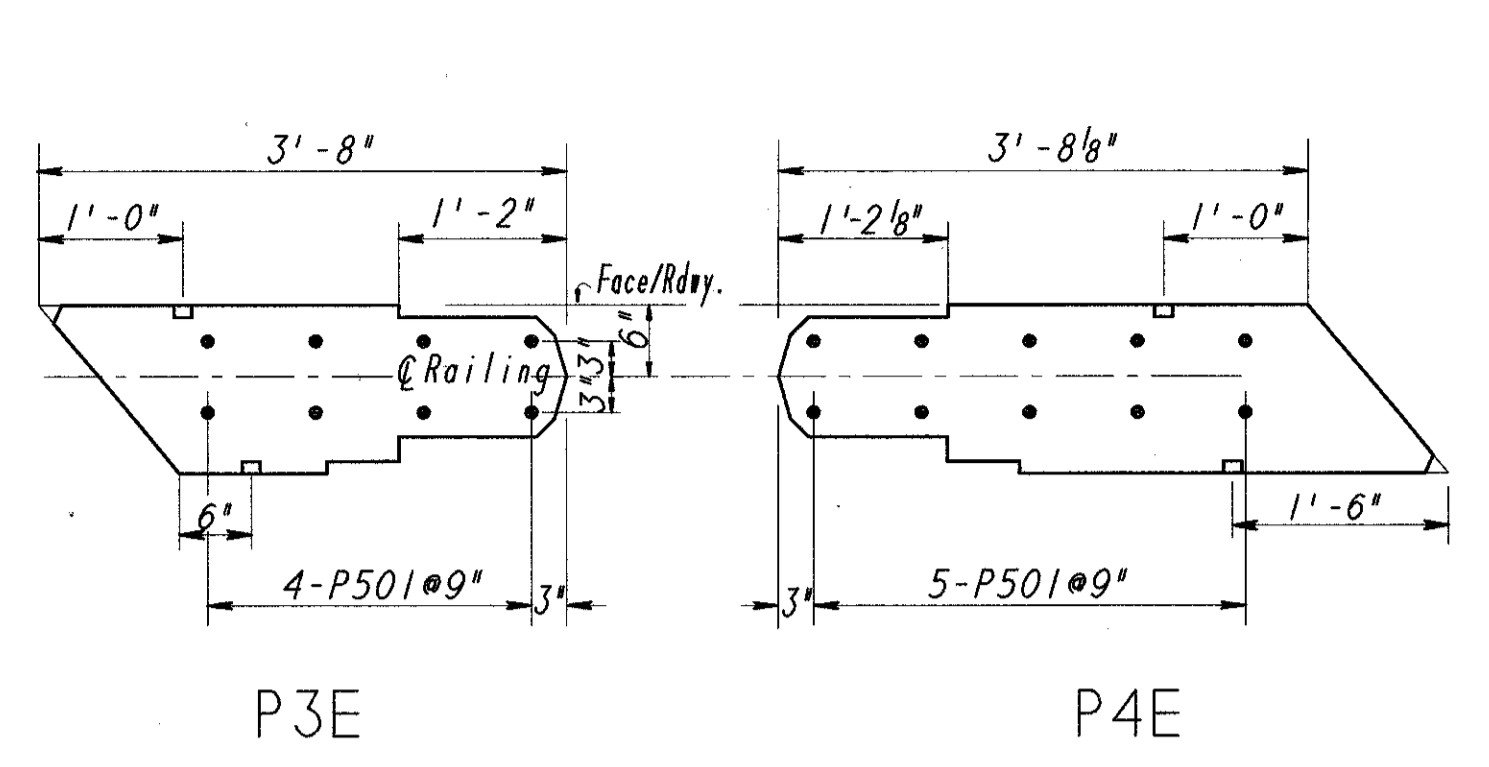
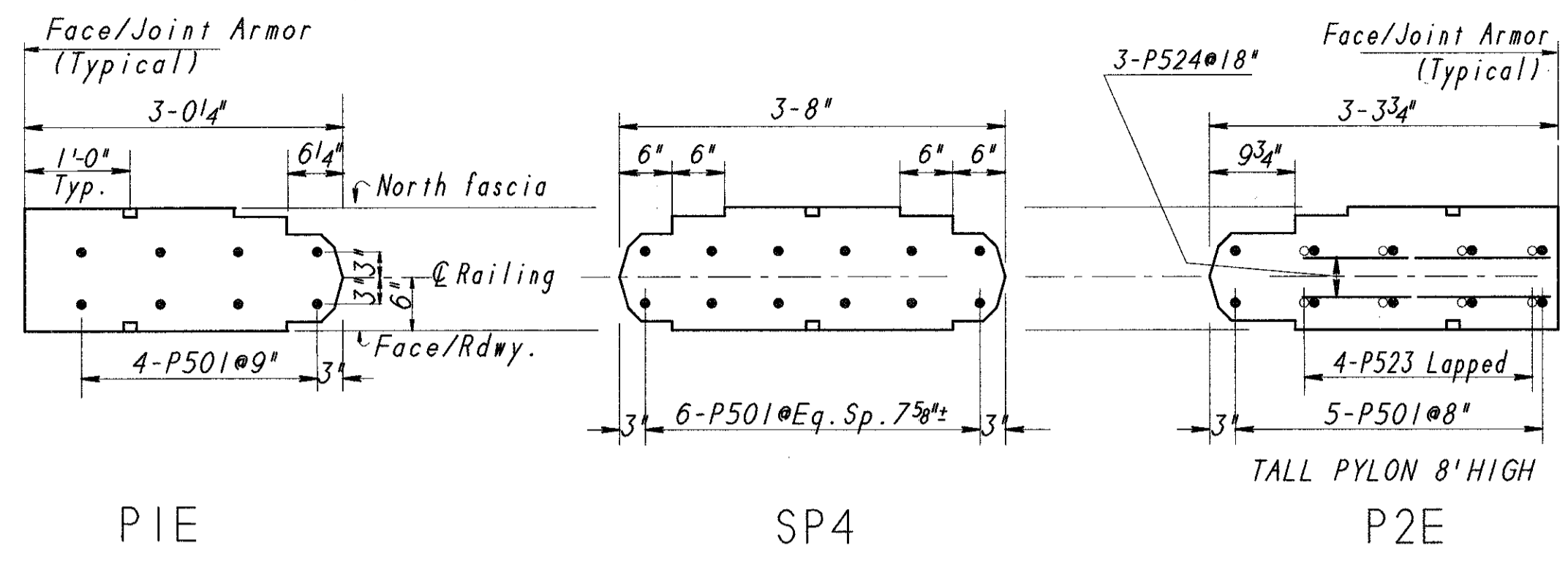
NOTES:

- 1.-Work this sheet with DECORATIVE RAILING SPANS 1-2 THRU 4-5 (DECK SLAB UNITS A,B,C & D), Sh. [60/108] [61/108] [62/108] [63/108]
- 2.-Work this sheet with GENERAL PLAN 1/3 Sh. [4/108] with SPAN 1-2 DECK SLAB UNIT A, Sh. [60/108] with SPAN 2-3 DECK SLAB UNIT B, Sh. [61/108] with SPAN 3-4 DECK SLAB UNIT C, Sh. [62/108] with SPAN 3-5 DECK SLAB UNIT D, Sh. [63/108]
- 3.-Length of pylons are measured along the same lines as shown on Deck Slab Plans.
- 4.-Placement of bar P501 within End Pylons shall be as detailed herein. Spacing for bars P501 between End Pylons shall be as detailed on Pylon Side Elevation, this sheet.
- 5.-For additional railing details not shown or referred to on this sheet, see DECORATIVE RAILING MISCELLANEOUS DETAILS, Sh. [76/108]
- 6.-For additional Notes and Legend, see Sh. [77/108]
- 7.-Pylons acute corner shall be Chamfered as Shown.
- 8.- See GENERAL NOTES 5/5 Sh. [111/108].

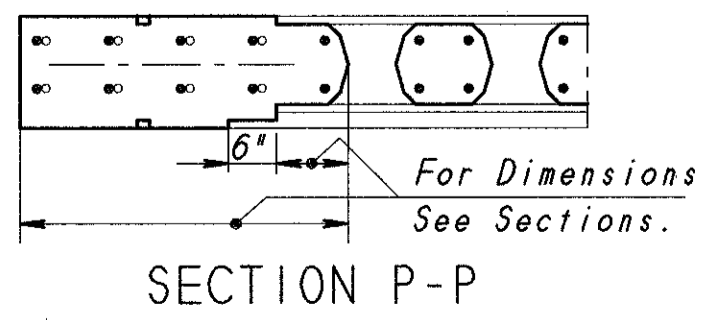
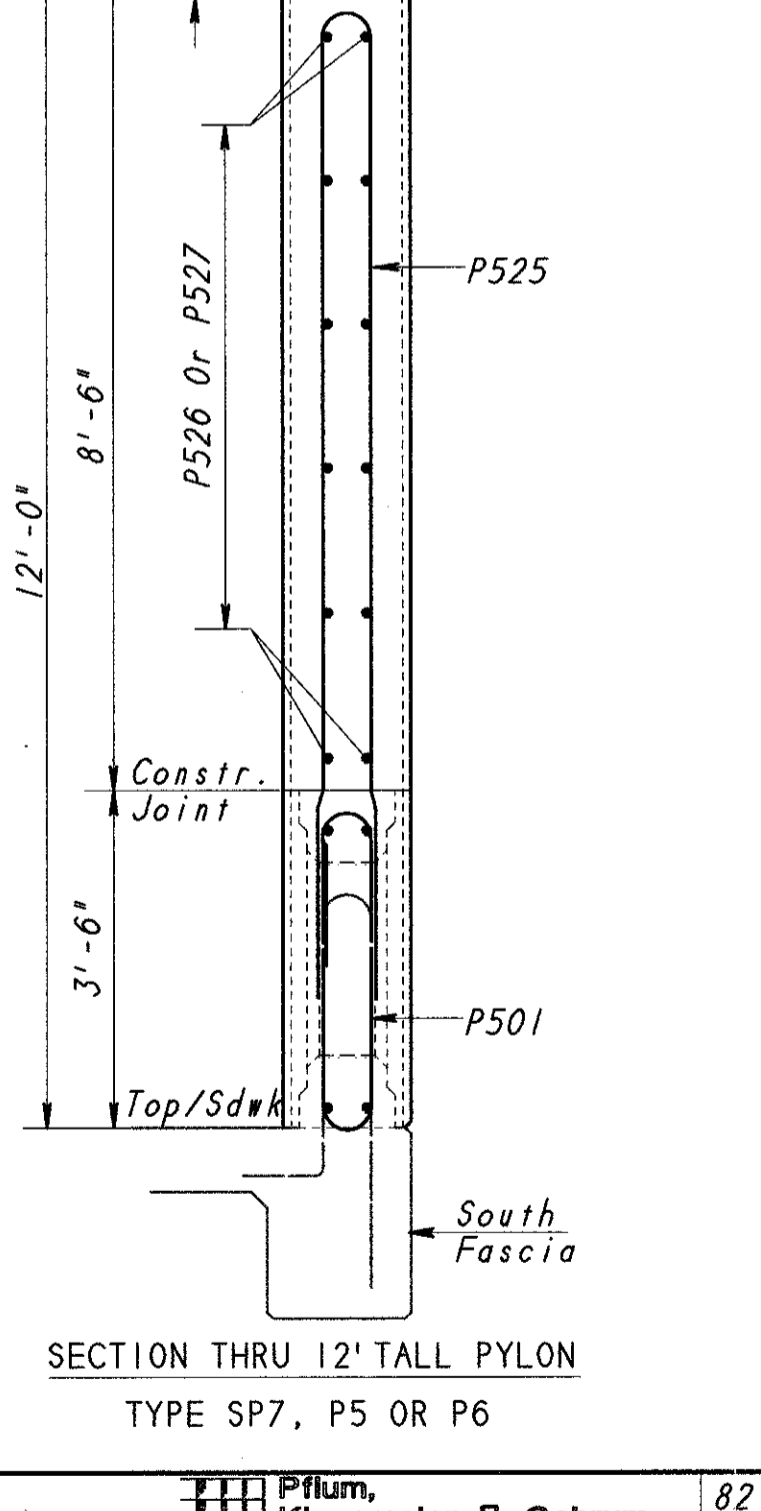
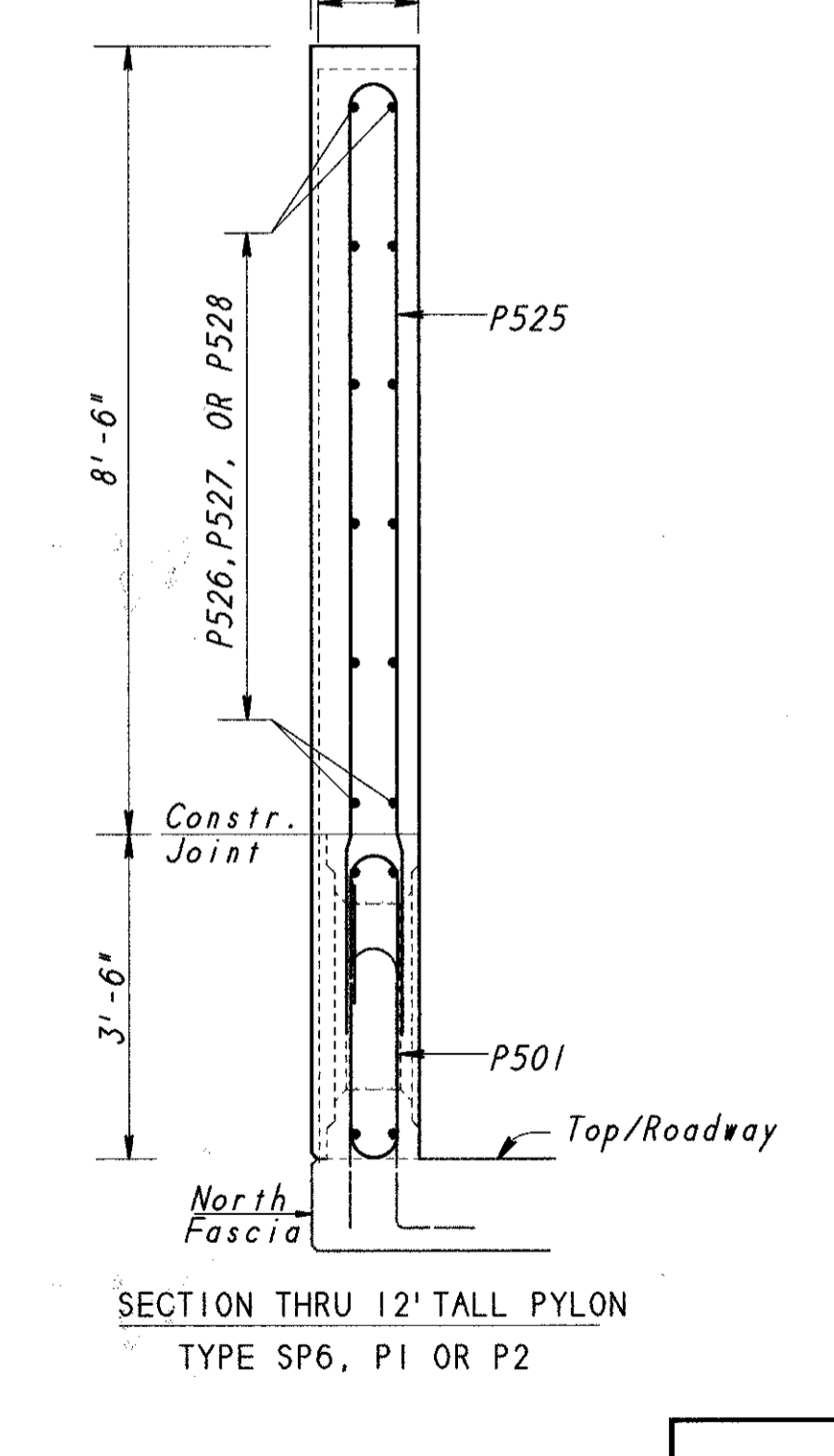
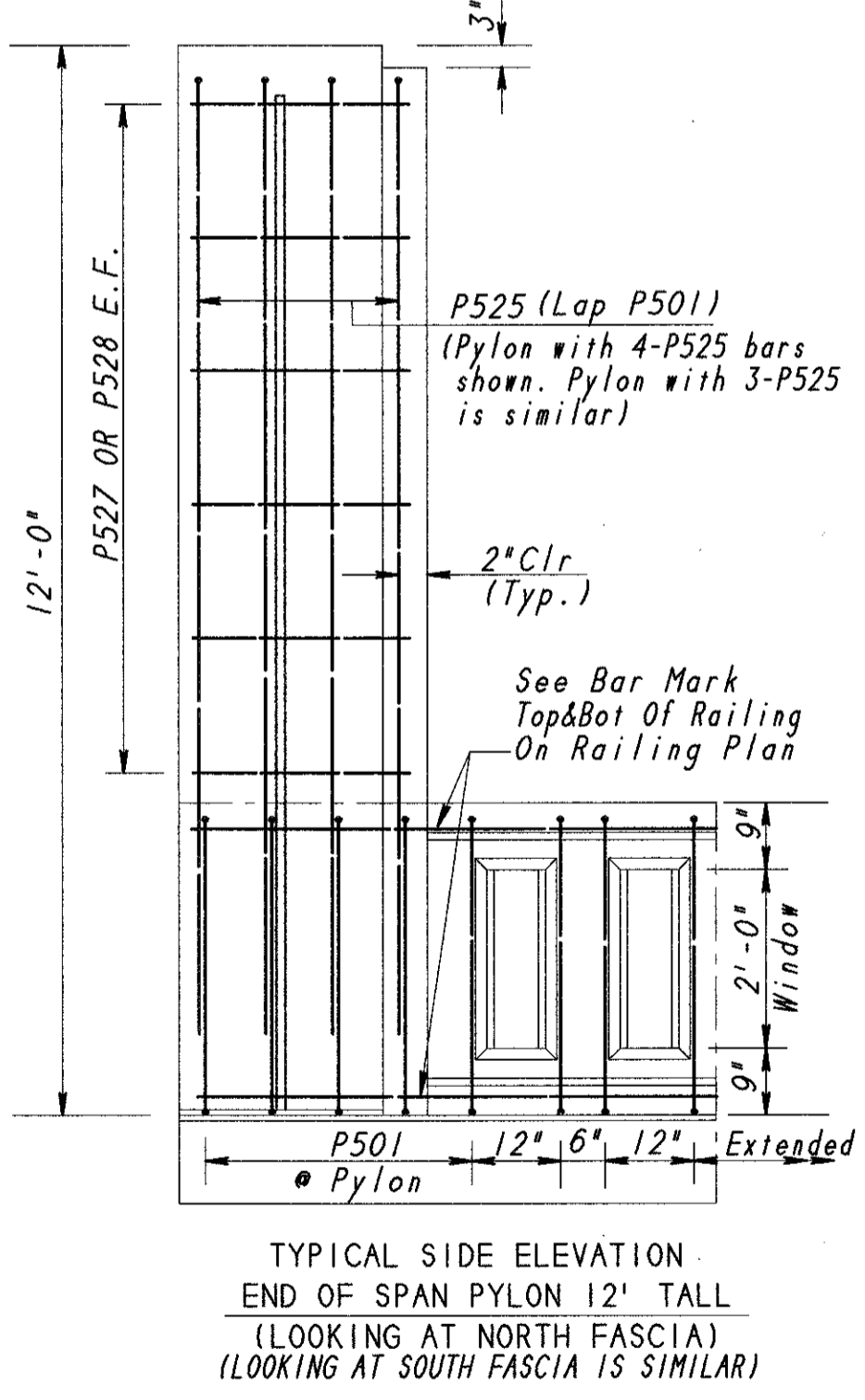
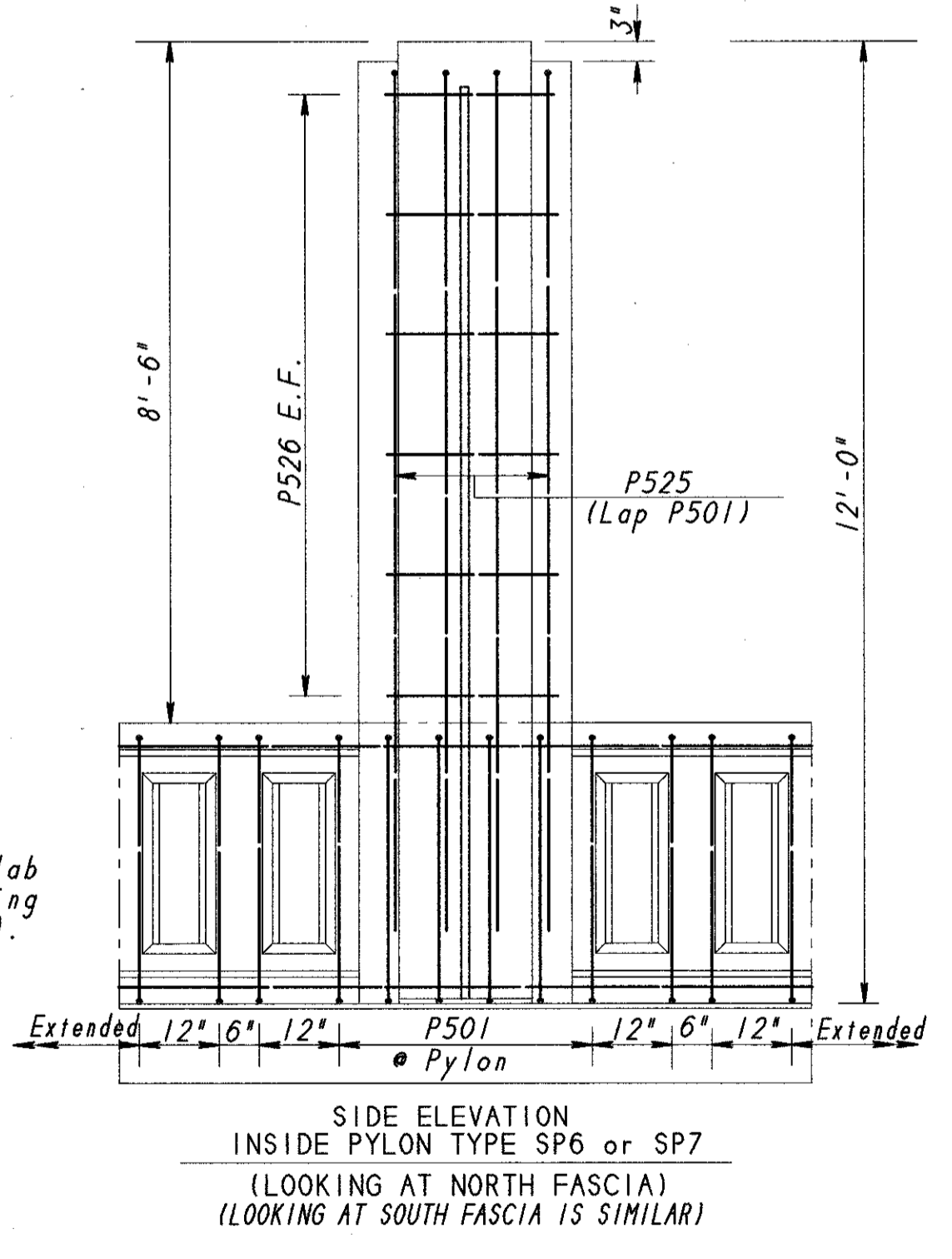
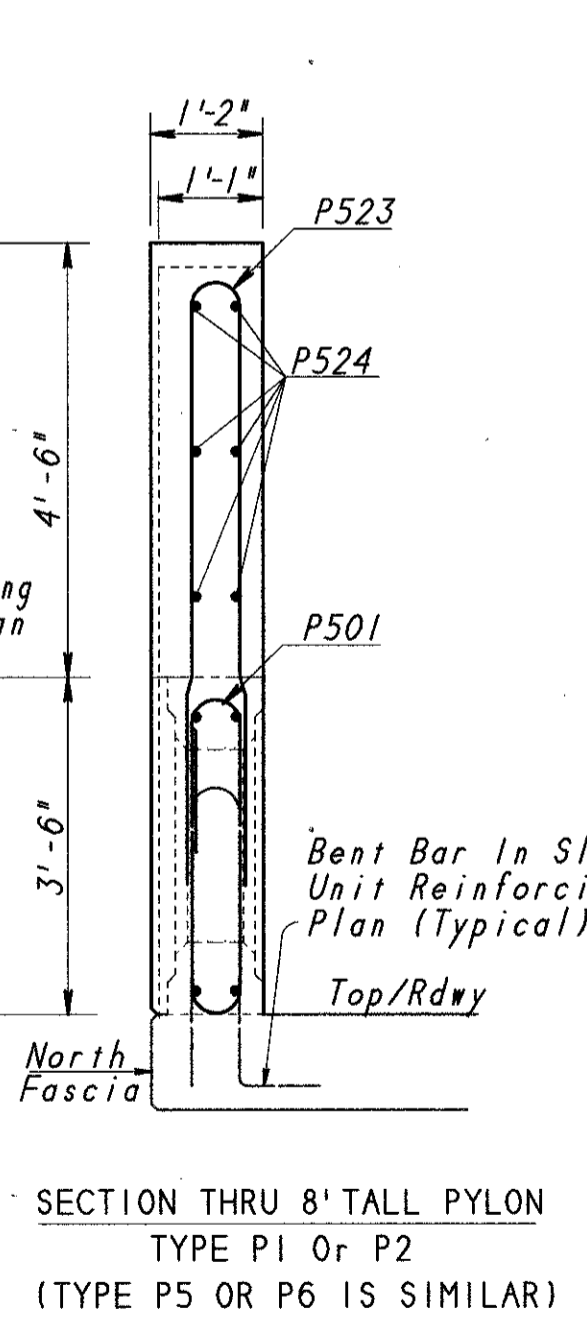
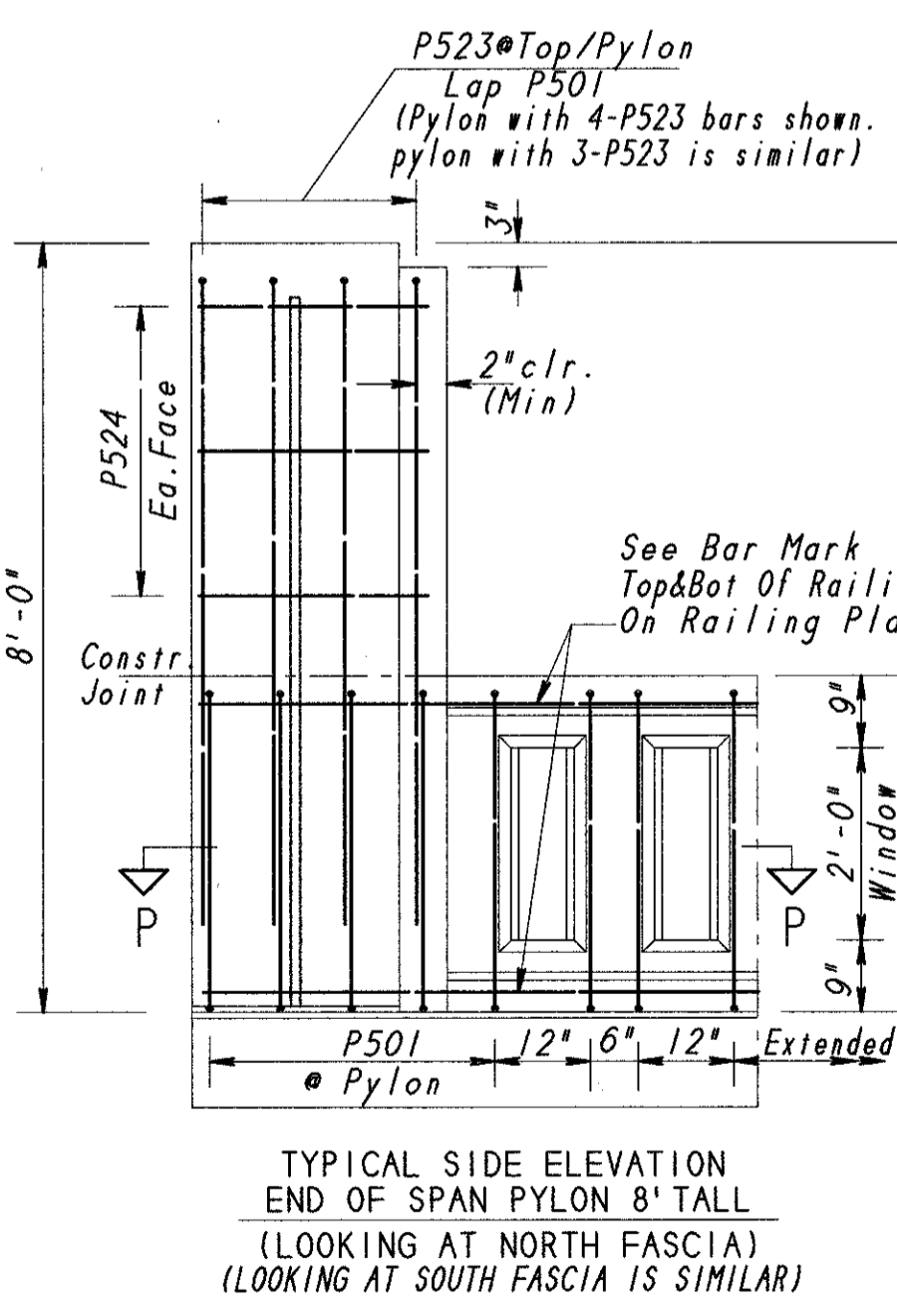
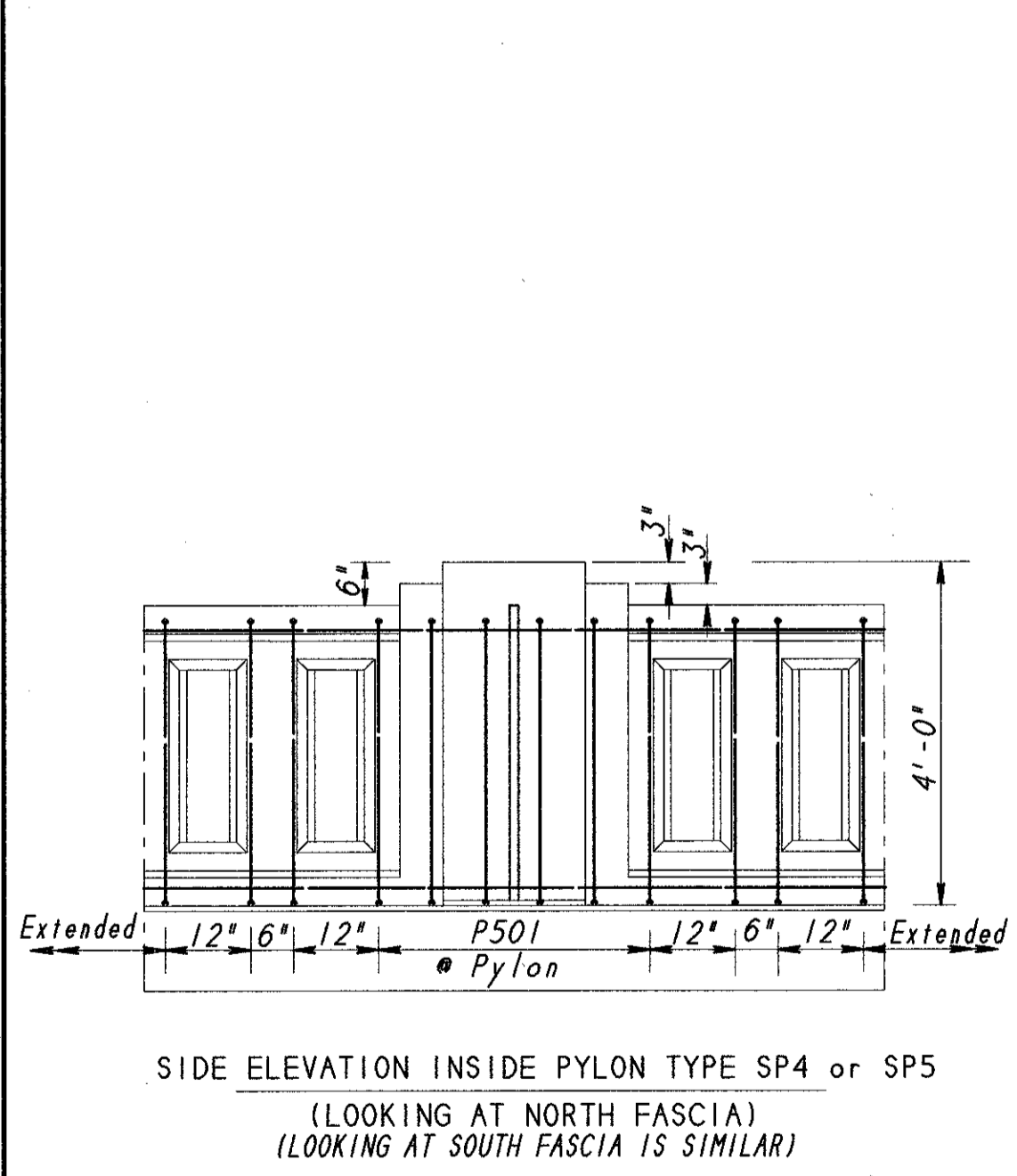


 Pflum, Klausmeier & Gehrmann Consultants		81/108 OHIO
DECORATIVE RAILING END OF SPAN PYLONS (SPAN 1-2 THRU 4-5) (SLAB UNITS A,B,C & D) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	ED	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96
REVISED		

13333



NOTE: Pylons not noted as TALL are normal railing height pylons.



NOTES:

1. - Work this sheet with DECORATIVE RAILING SPANS 5-6-7 & 7-8-9-10 (UNIT E & UNIT F) Sh. [78/108]
2. - Work this sheet with GENERAL PLAN 1/3 to 3/3 Sh. [4/108], Sh. [5/108] and Sh. [6/108] with DECK SLAB UNIT E, Sh. [64/108] with DECK SLAB UNIT F, Sh. [65/108] and Sh. [66/108] with DECK SLAB UNIT G, Sh. [67/108]

3. - Length of railing elements are measured along the same lines as shown in Deck Slab Plans.
4. - For additional Notes and Legend, see Sh. [77/108]
5. - For additional details not shown or referred to on this sheet, see DECORATIVE RAILING MISCELLANEOUS DETAILS, Sh. [76/108]
6. - See GENERAL NOTES 5/5 Sh. [77/108]

Plum, Klausmeier & Gehrm
Consultants

DECORATIVE RAILING
END OF SPAN PYLONS
(SPANS 5-6-7 & 7-8-9-10)
(SLAB UNITS E & F)
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY

DESIGNED ED DRAWN ED TRACED WDD CHECKED WDD REVIEWED IEH DATE April 96

82/108

Sta. 30+55.40
Sta. 47+16.19

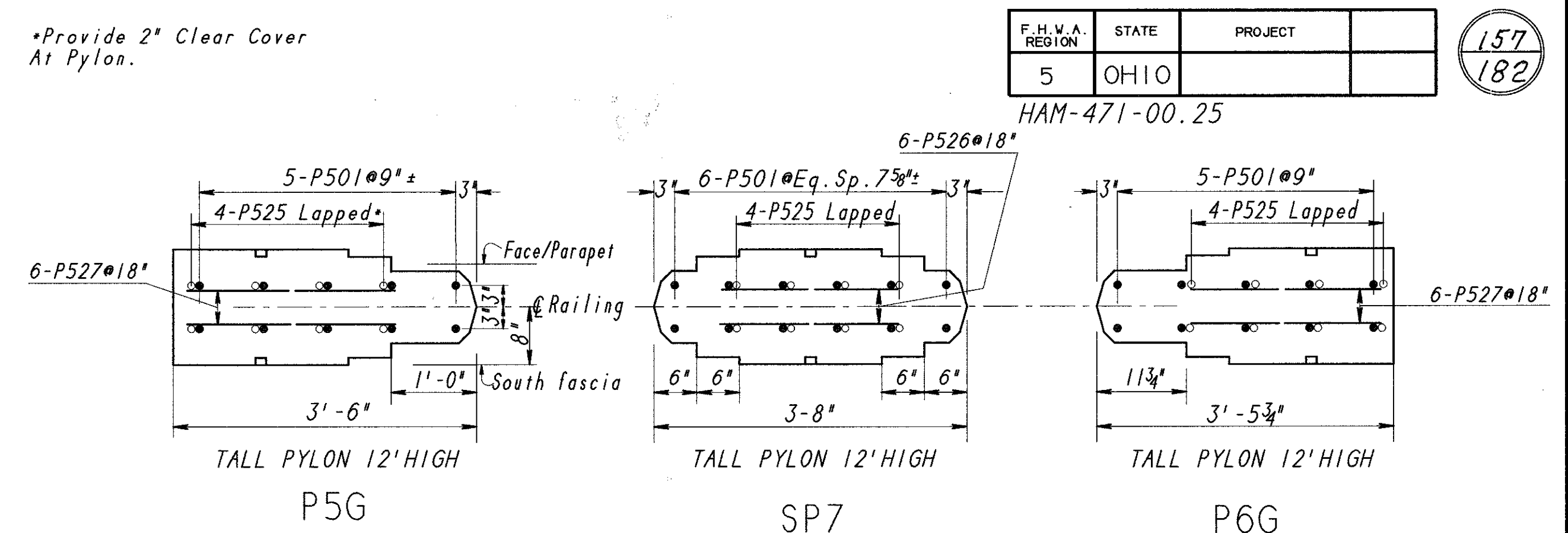
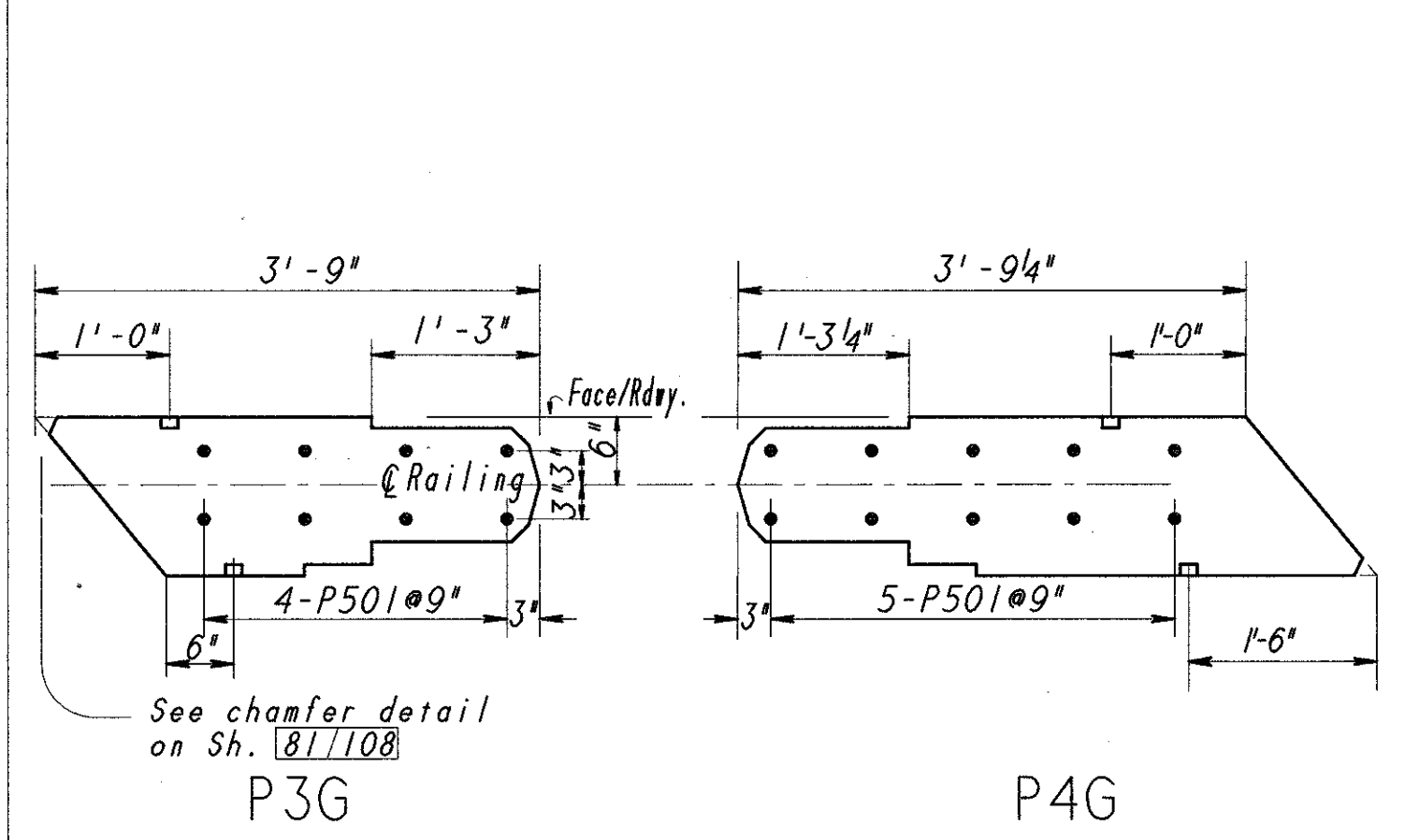
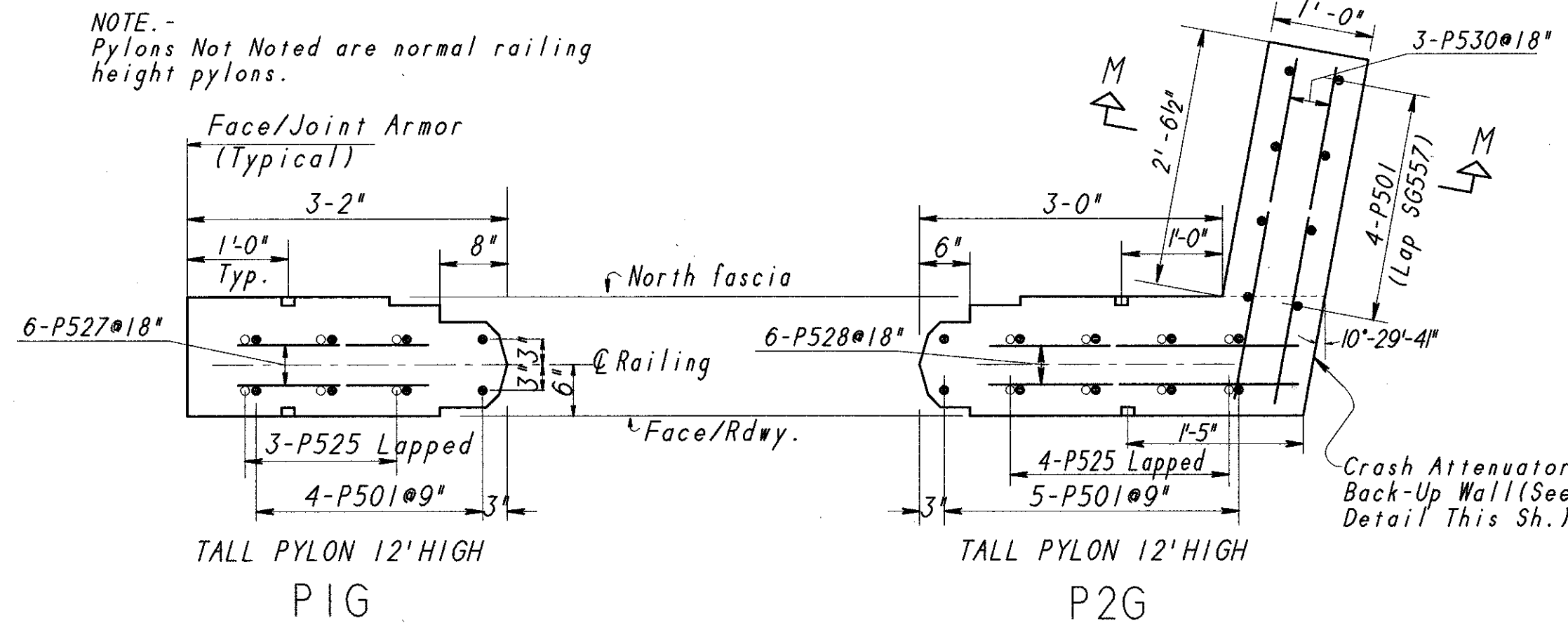
TYPICAL Scale 1/32

NOTE: - Pylons Not Noted are normal railing height pylons.

F. H. V. A. REGION	STATE	PROJECT
5	OHIO	

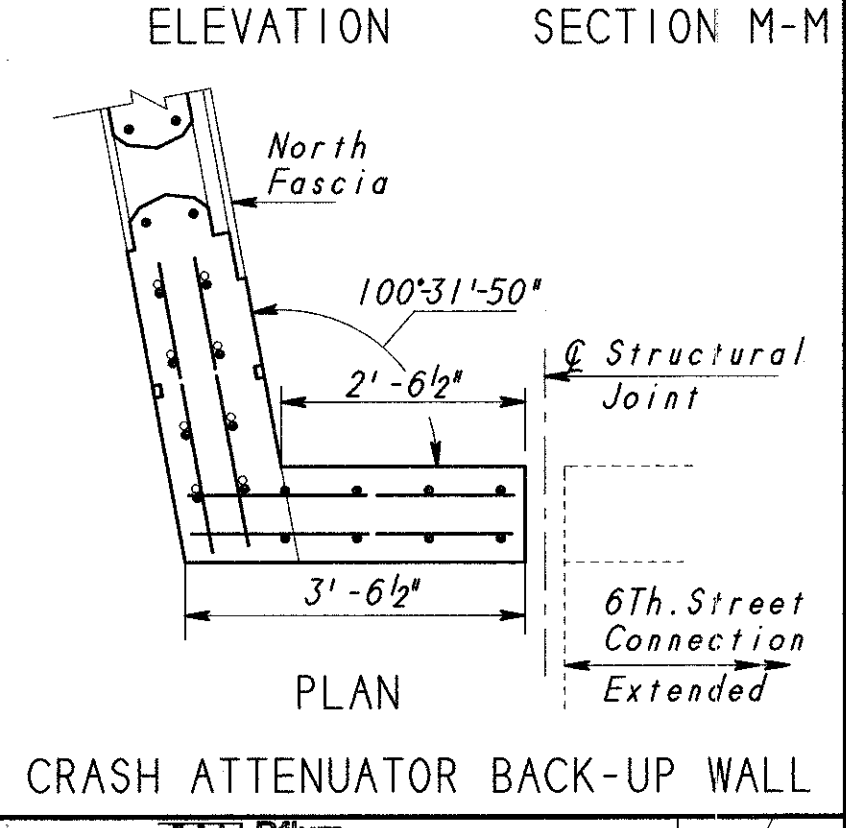
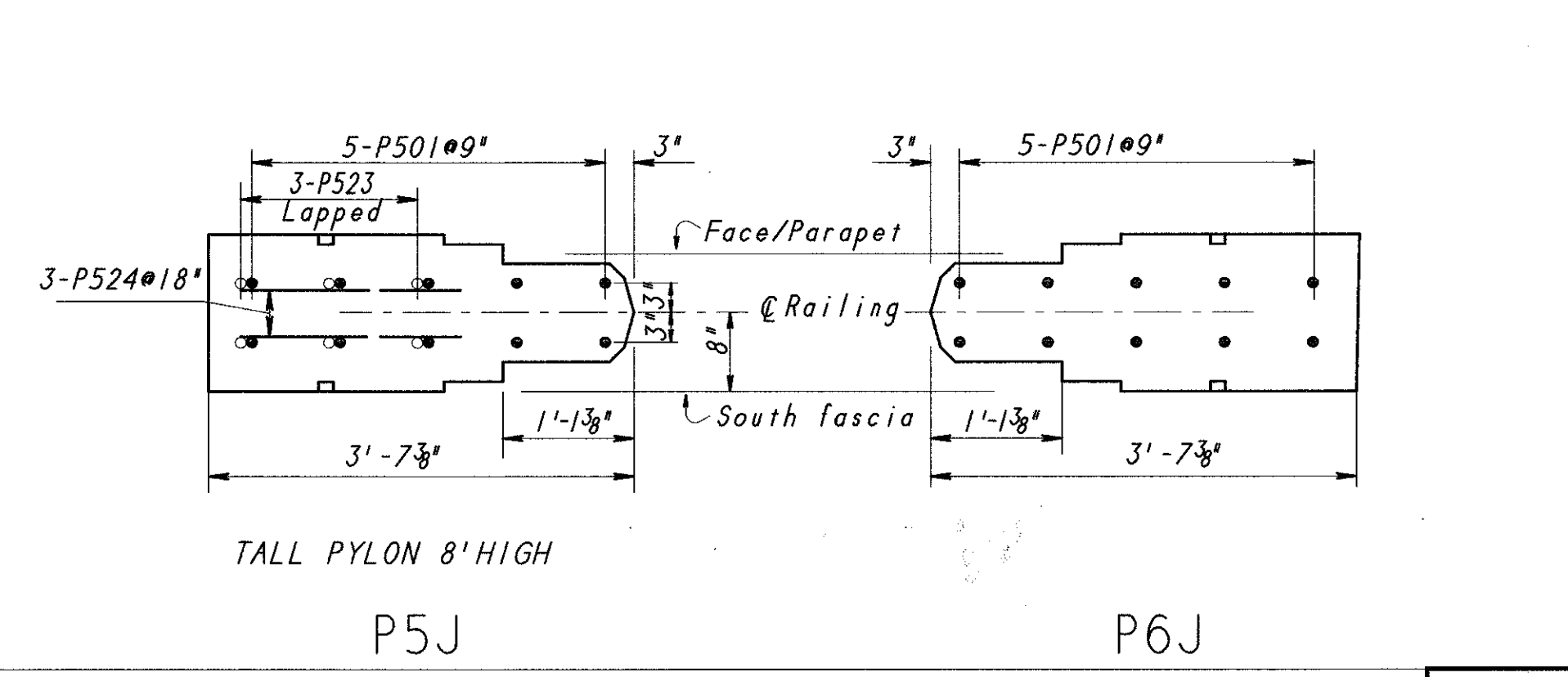
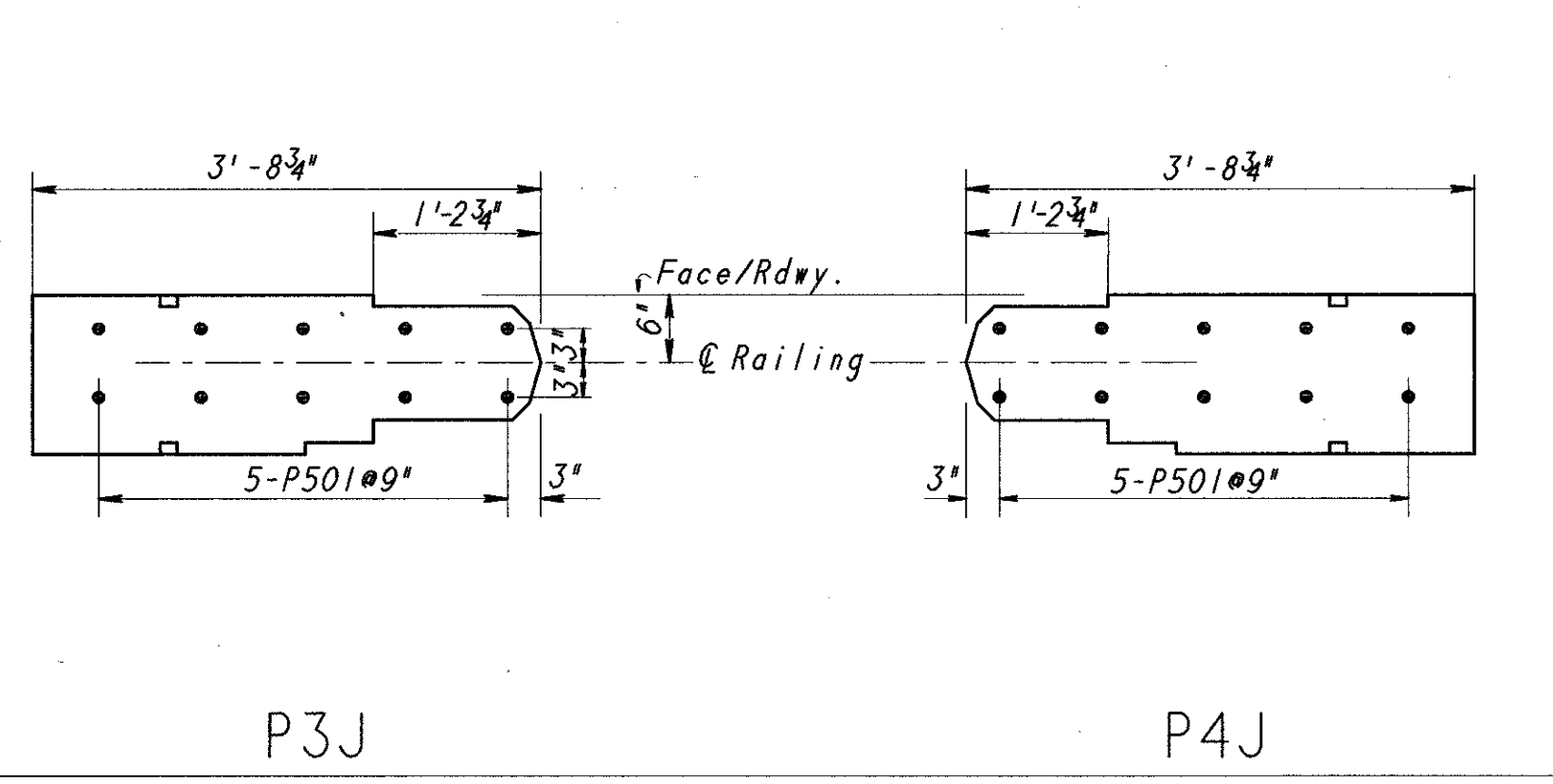
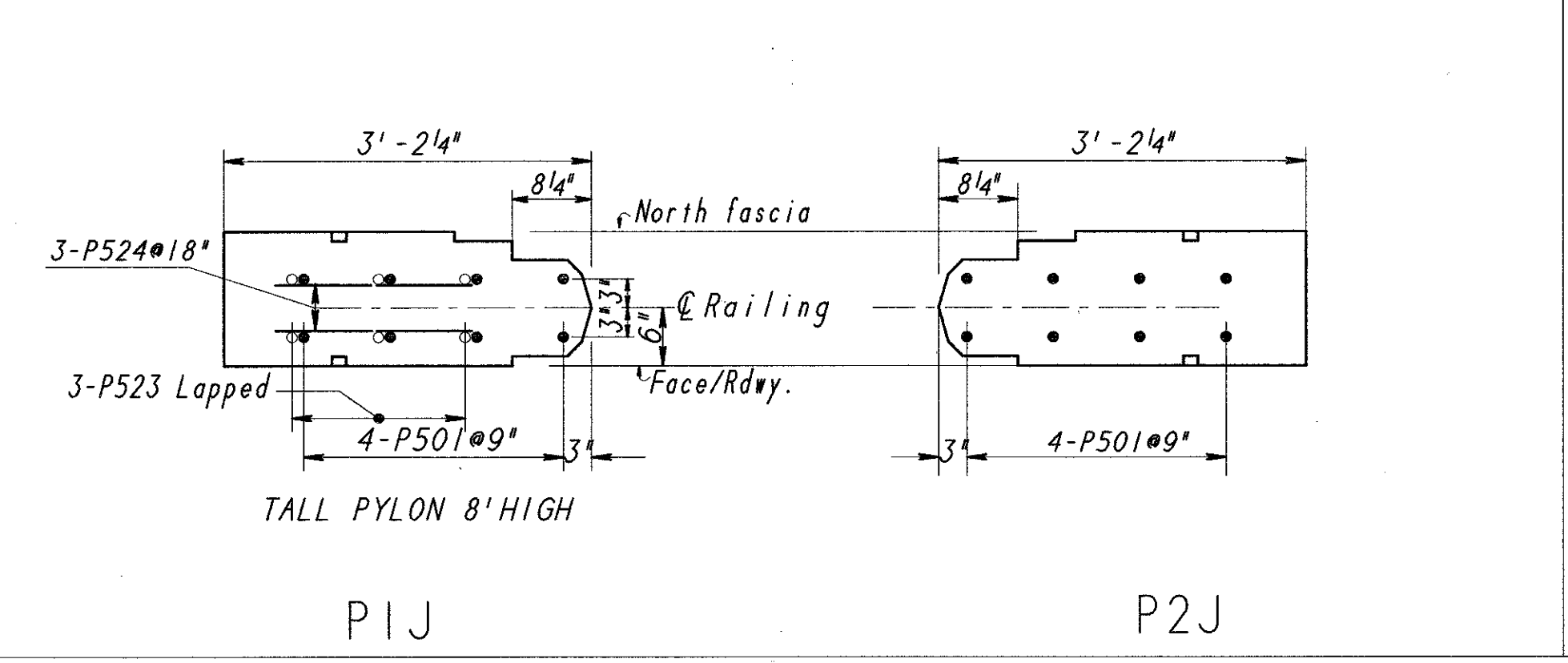
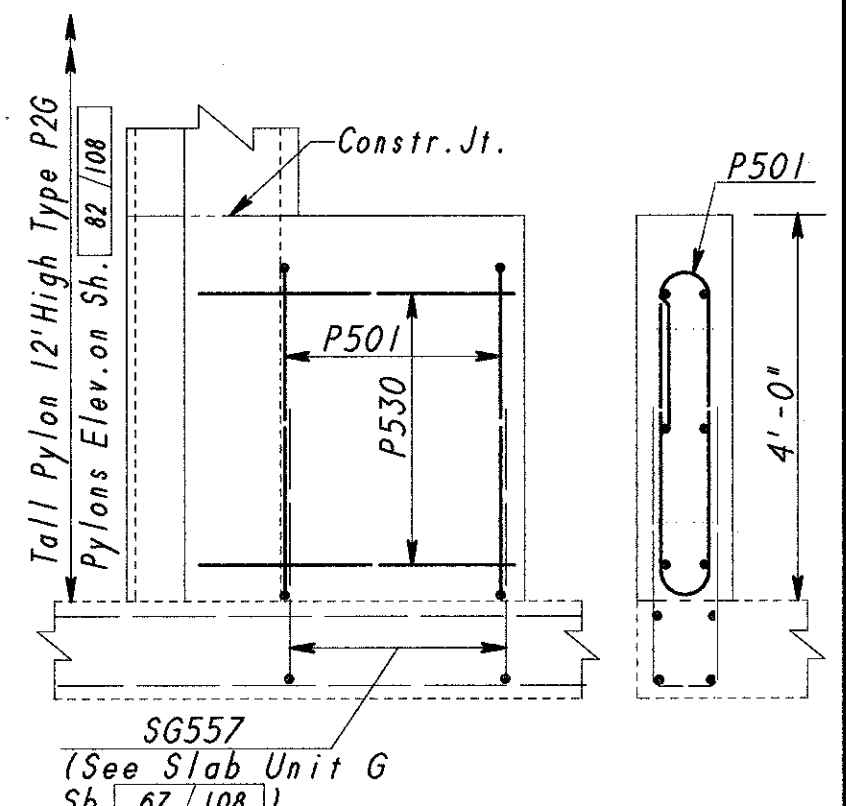
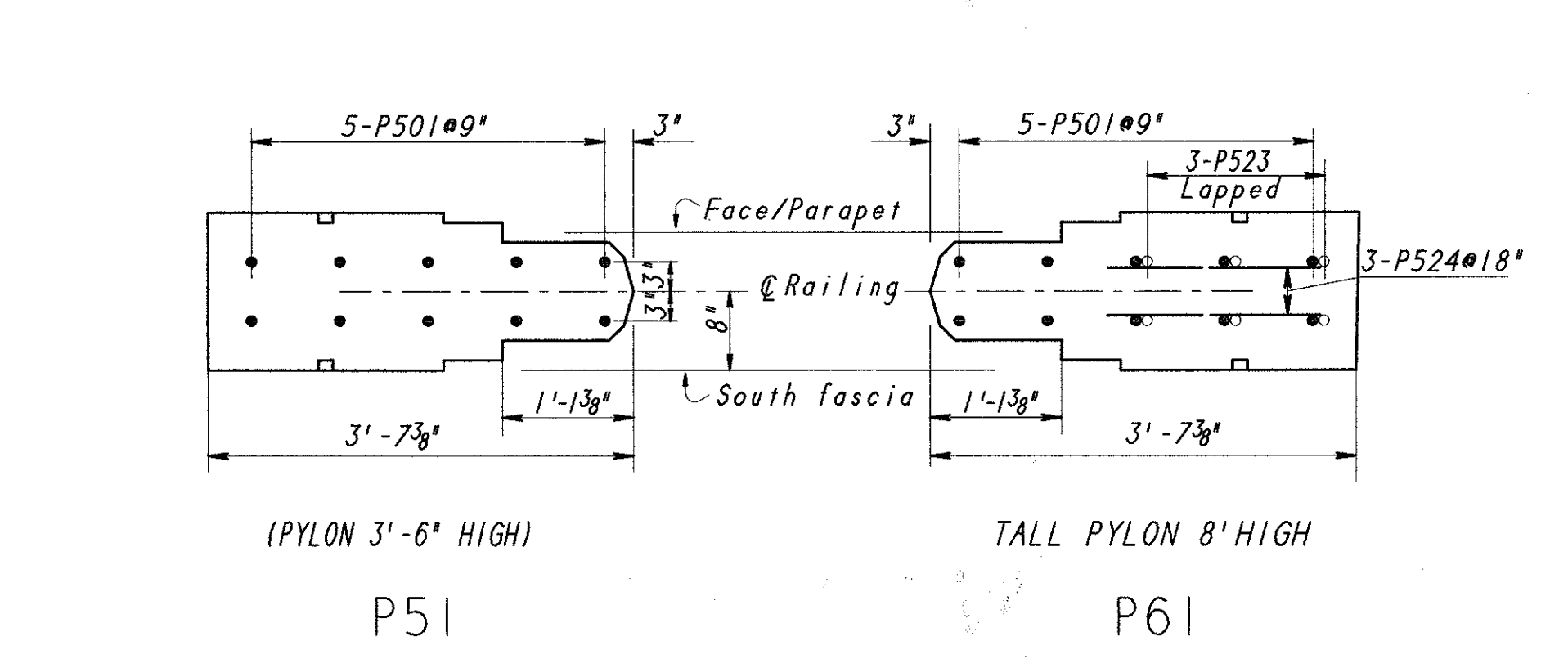
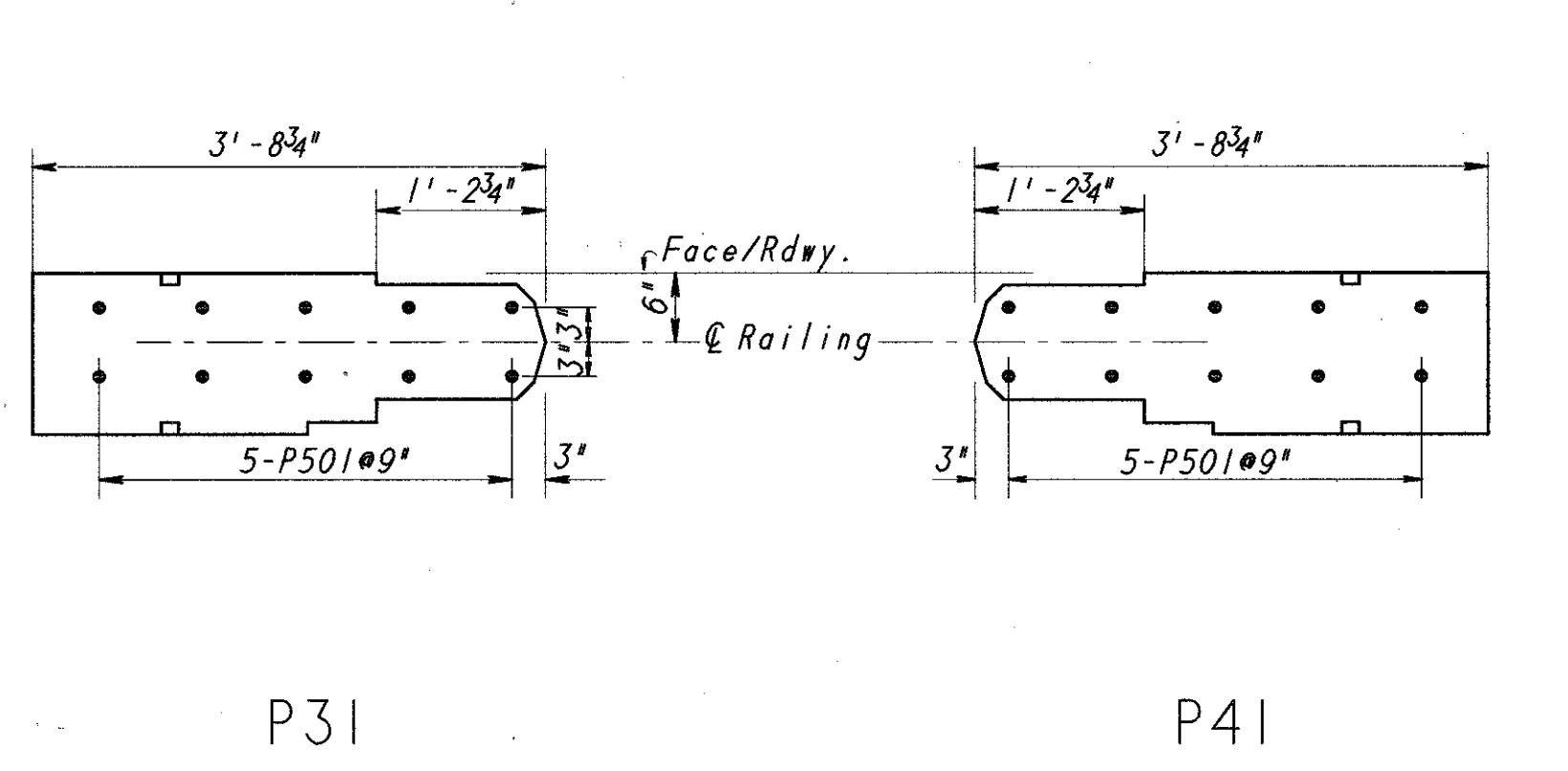
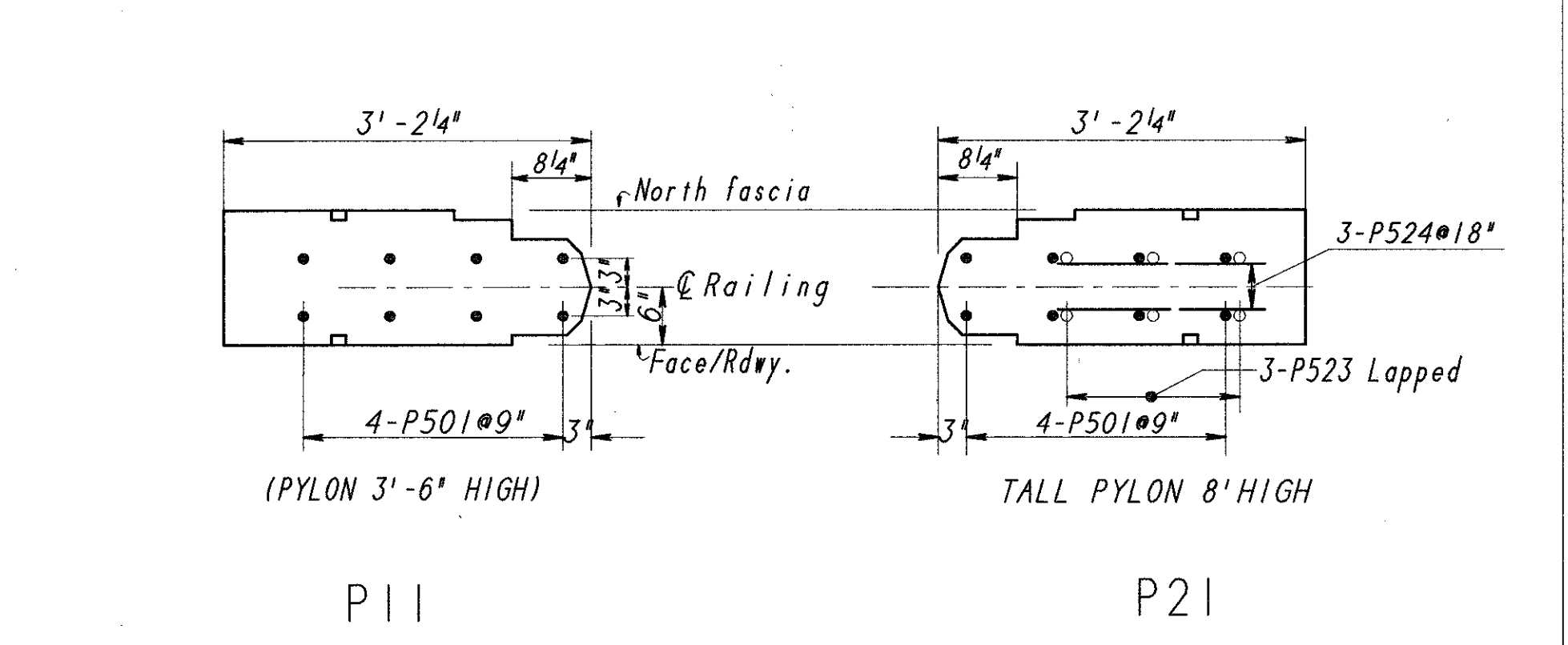
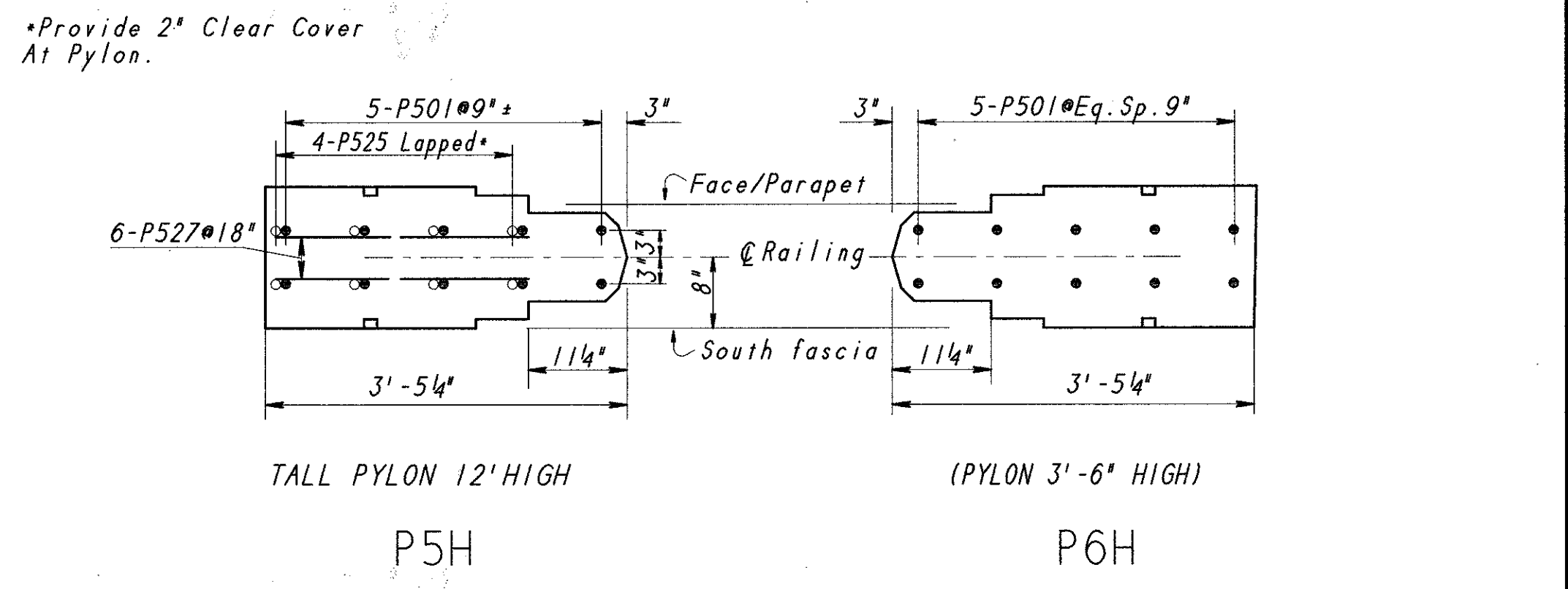
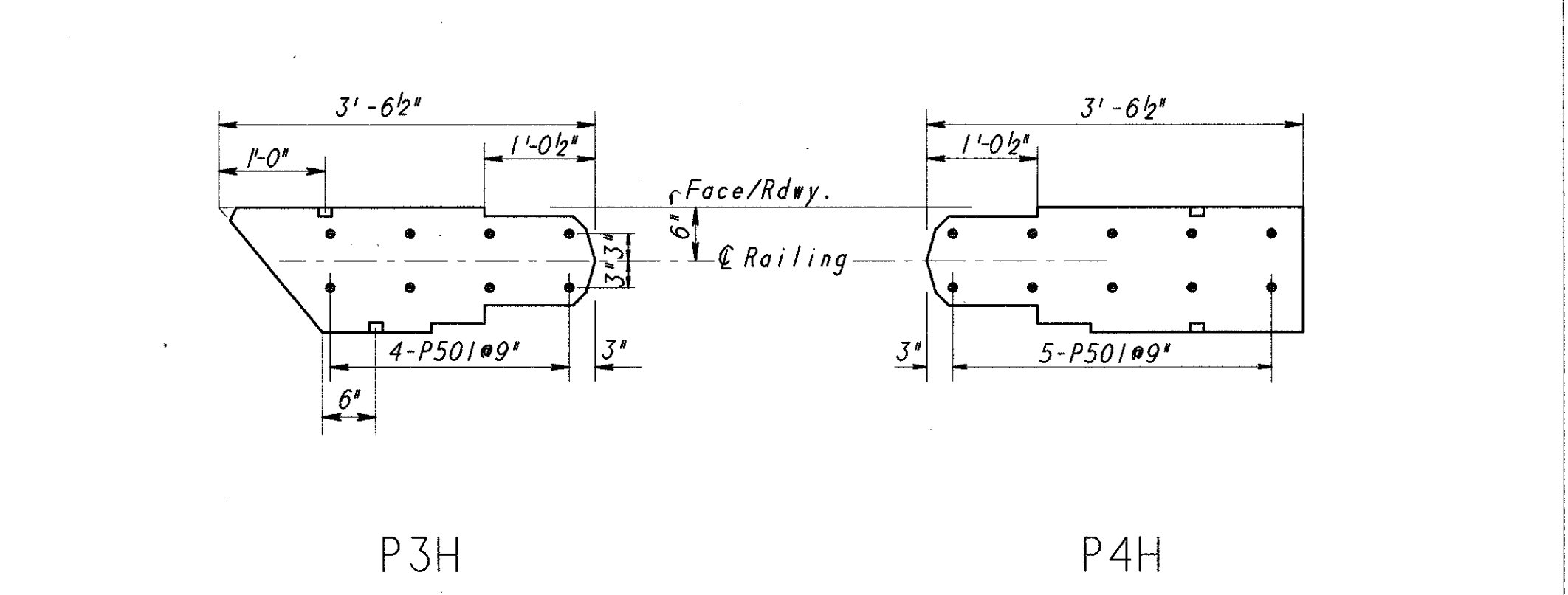
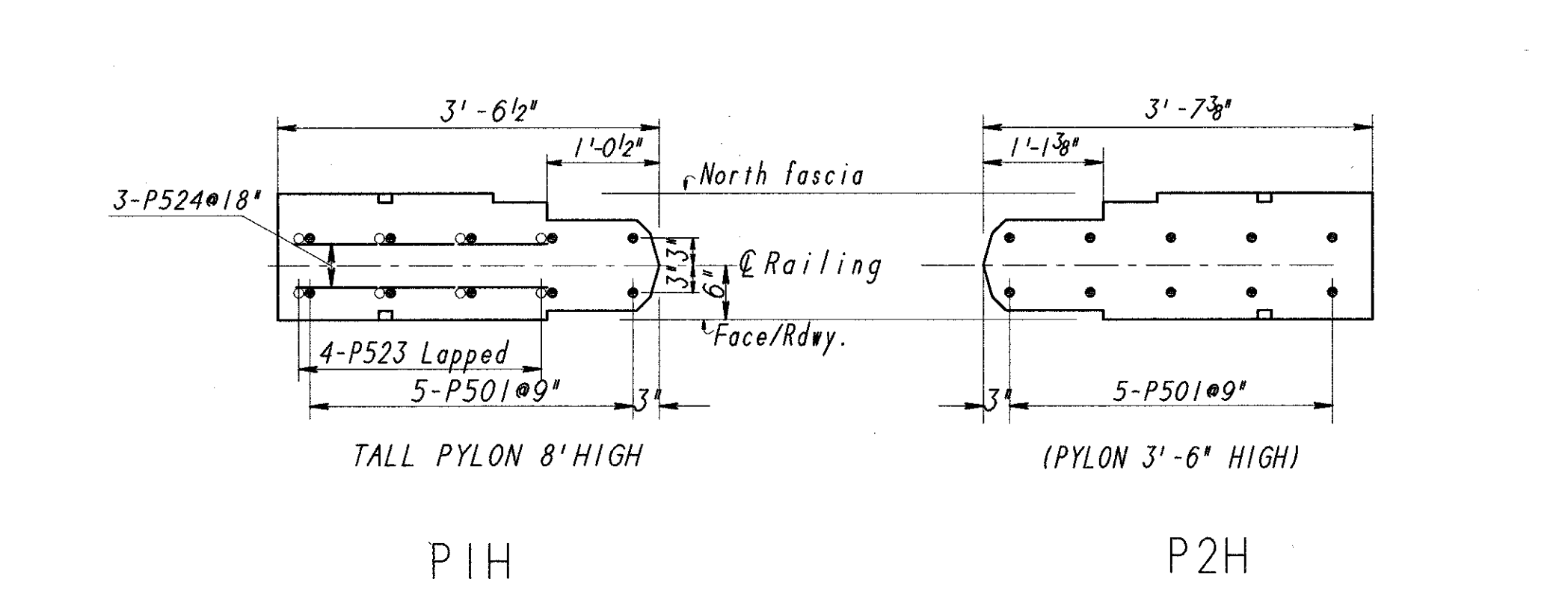
157
182

HAM-471-00.25



*Provide 2' Clear Cover At Pylon.

*Provide 2' Clear Cover At Pylon.



NOTES:

- 1.-Work this sheet with DECORATIVE RAILING, SPANS 10-11-12, 12-13, 13-14 and 14-15 (Slab Units G, H, I & J), Sh. 79/108
- 2.-Work this sheet with GENERAL PLAN 1/3 to 3/3, Sh. 4/108, Sh. 5/108 and Sh. 6/108 with DECK SLAB UNIT G, Sh. 67/108 with DECK SLAB UNIT H, Sh. 68/108 with DECK SLAB UNIT I, Sh. 69/108 with DECK SLAB UNIT J, Sh. 70/108
- 3.-For additional Notes and Legend, see Sh. 77/108 and Sh. 81/108
- 4.-See GENERAL NOTES 5/5, Sh. 1/1/108.

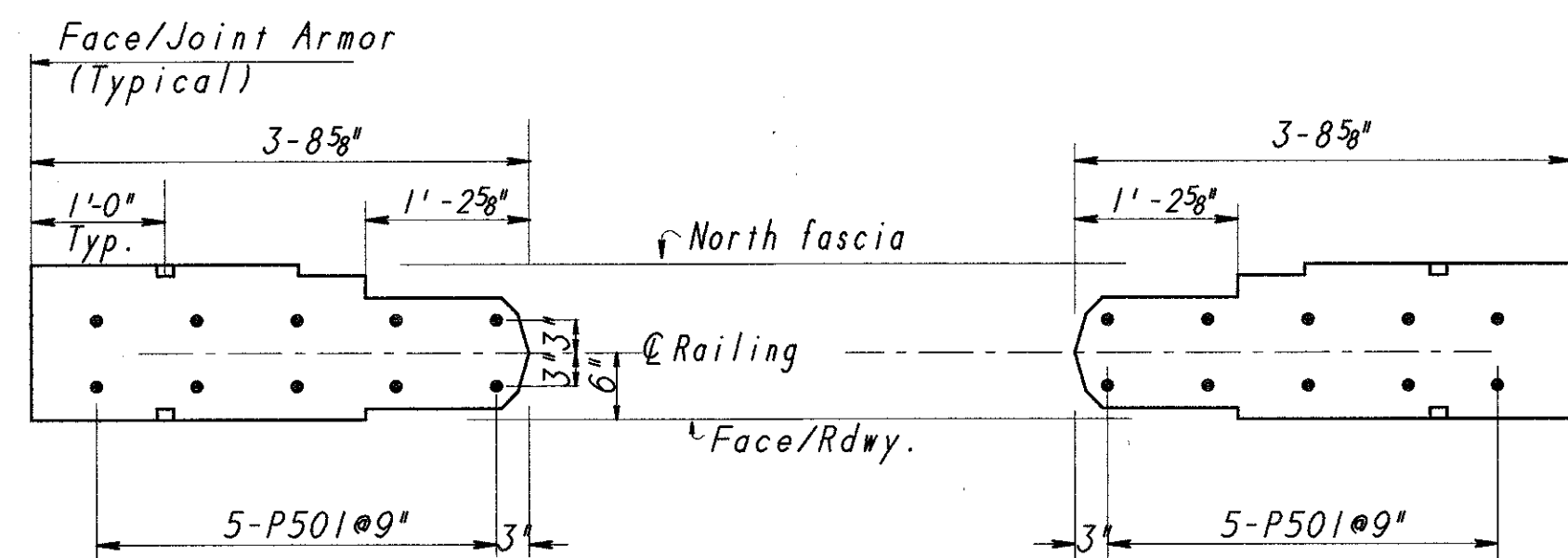
Plum, Klausmeyer & Gehrmann
CONSULTANTS
83/108

OHIO
DECORATIVE RAILING
END OF SPAN PYLONS
(SPANS 10-11-12, 12-13, 13-14 & 14-15)
(SLAB UNITS G, H, I & J)
BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

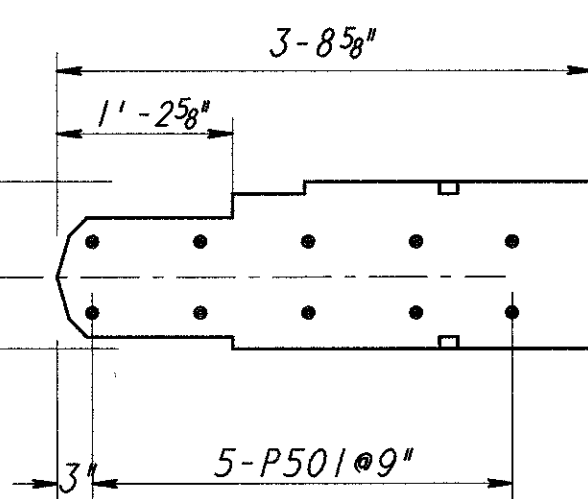
HAMILTON COUNTY
Sta. 30+55.40
Sta. 47+16.19

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED		WDD	IEH	April 96	

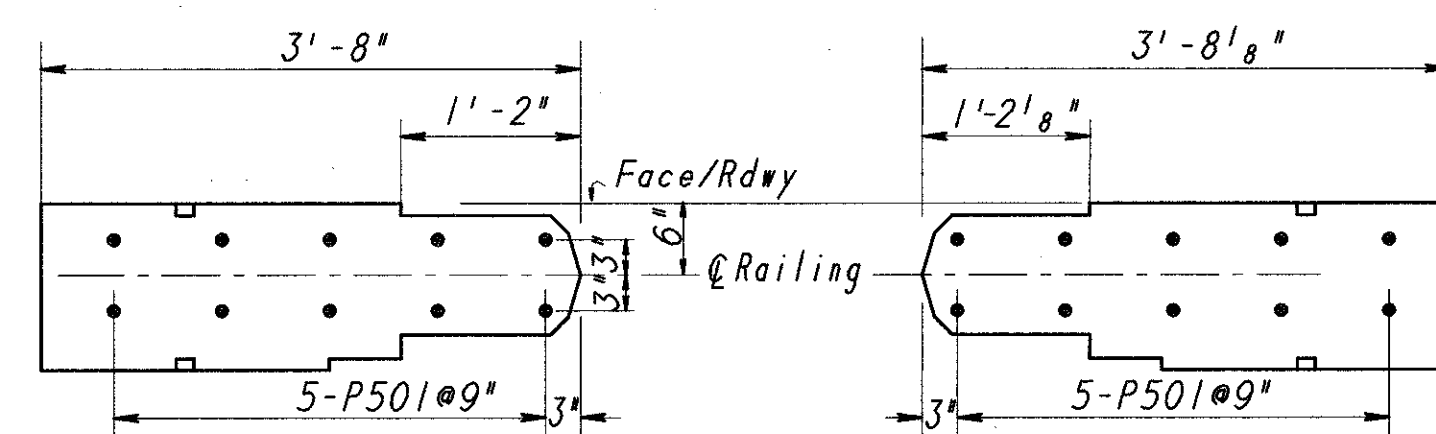
TAEPOH Scale 1:3333



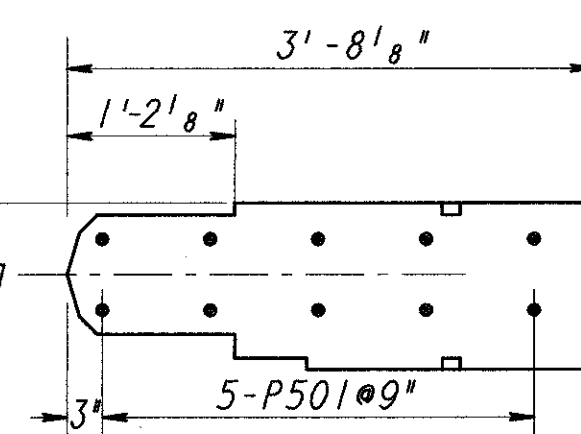
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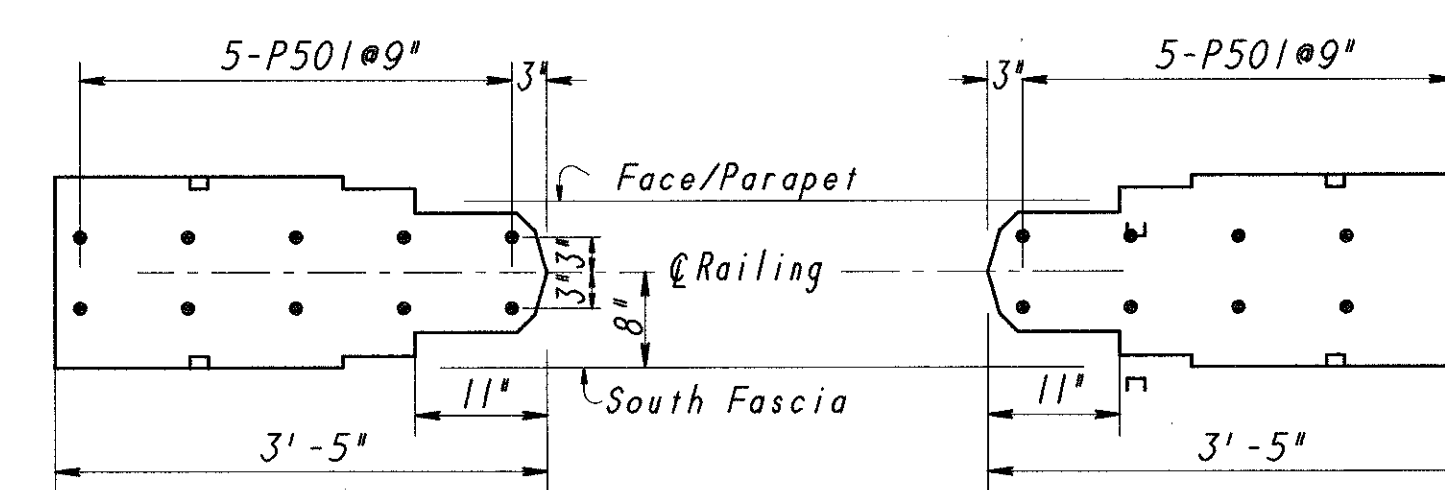
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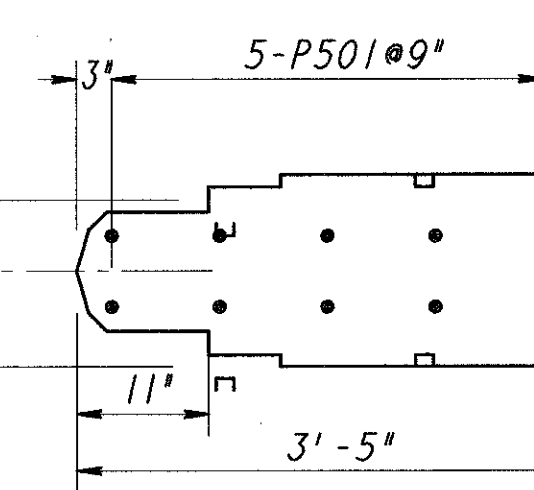
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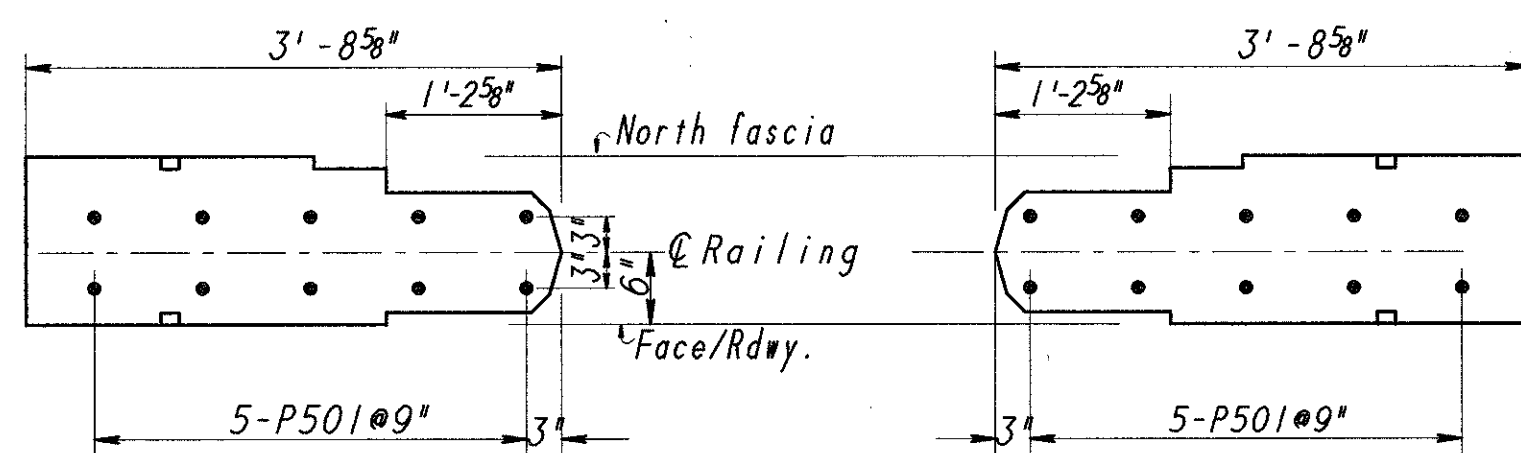
P4K



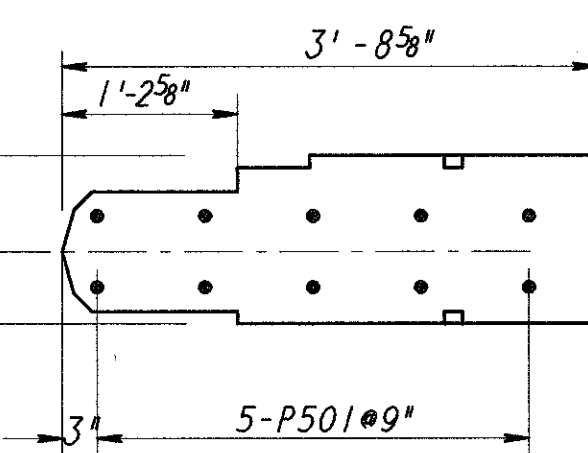
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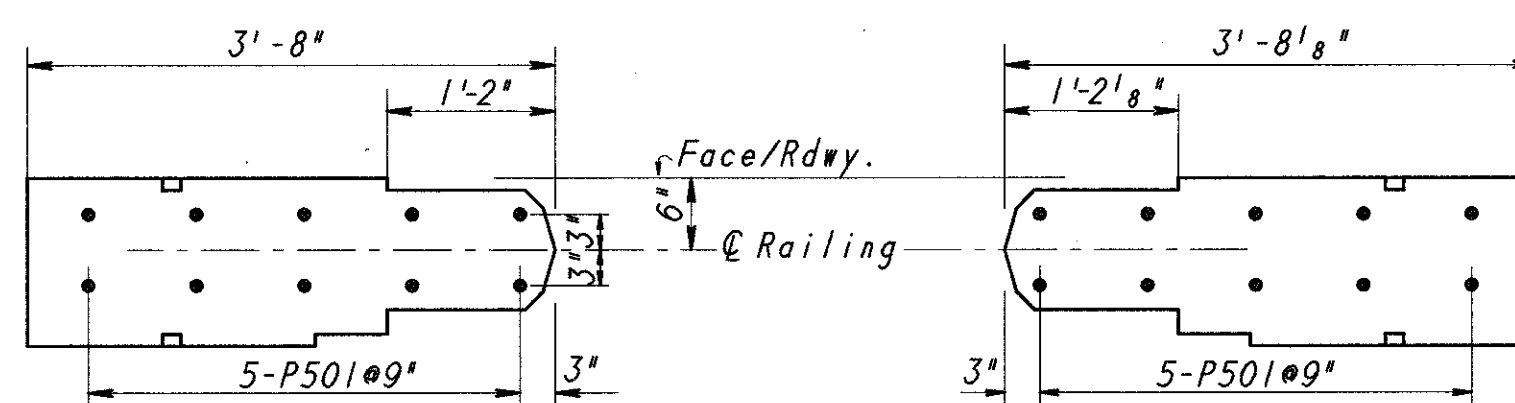
P6K



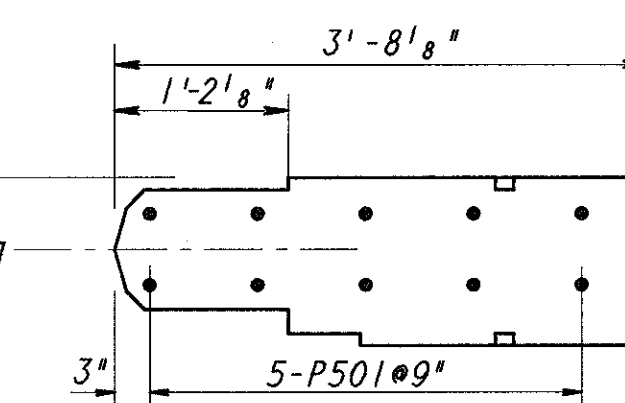
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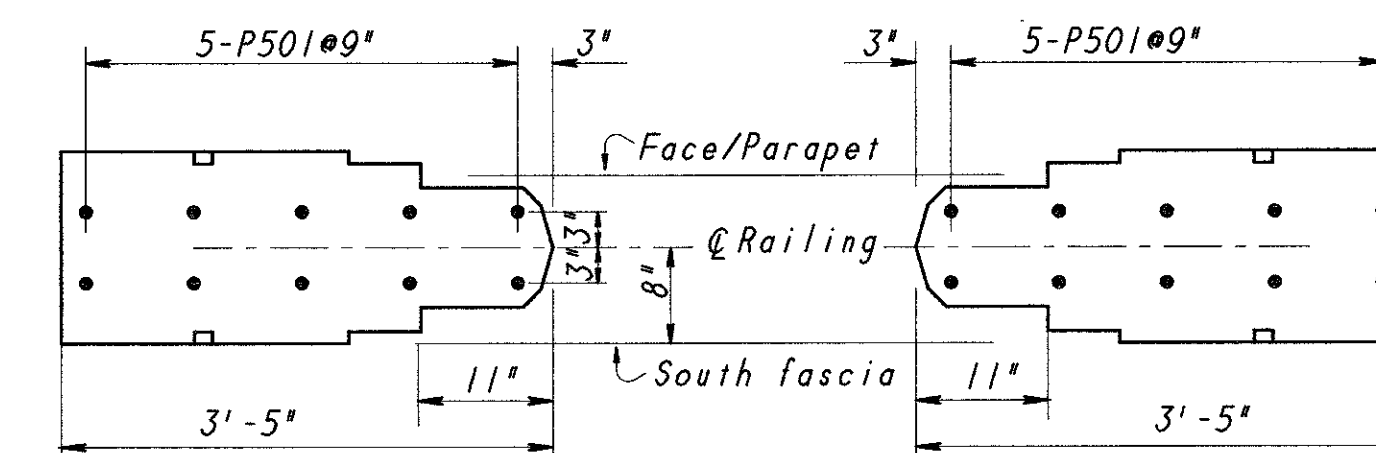
P2L



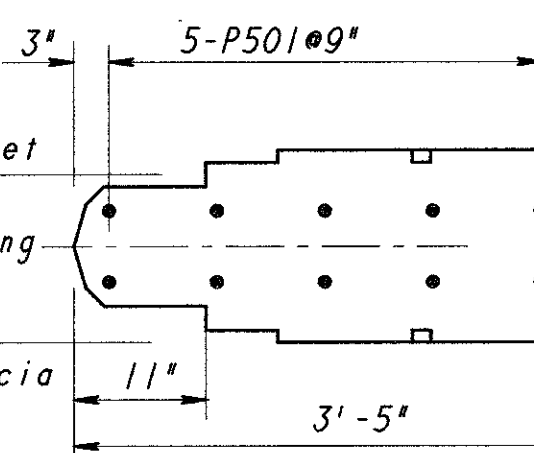
P3L



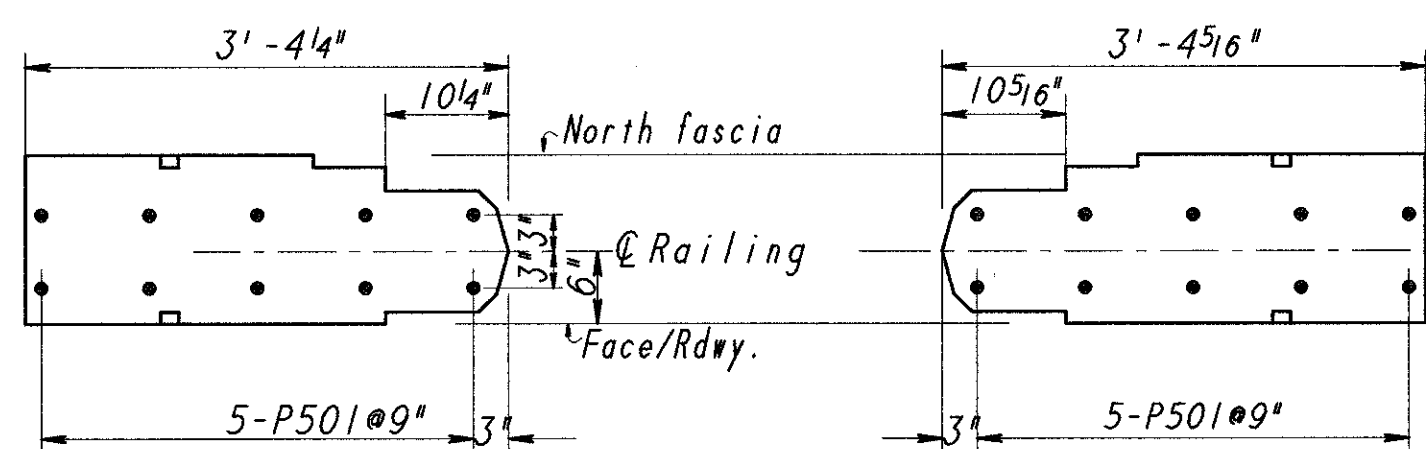
P4L



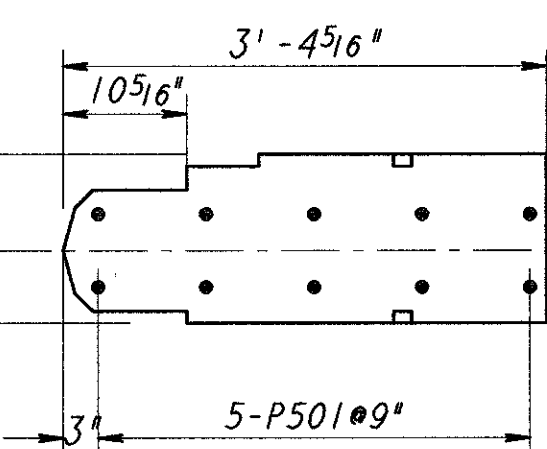
P5L



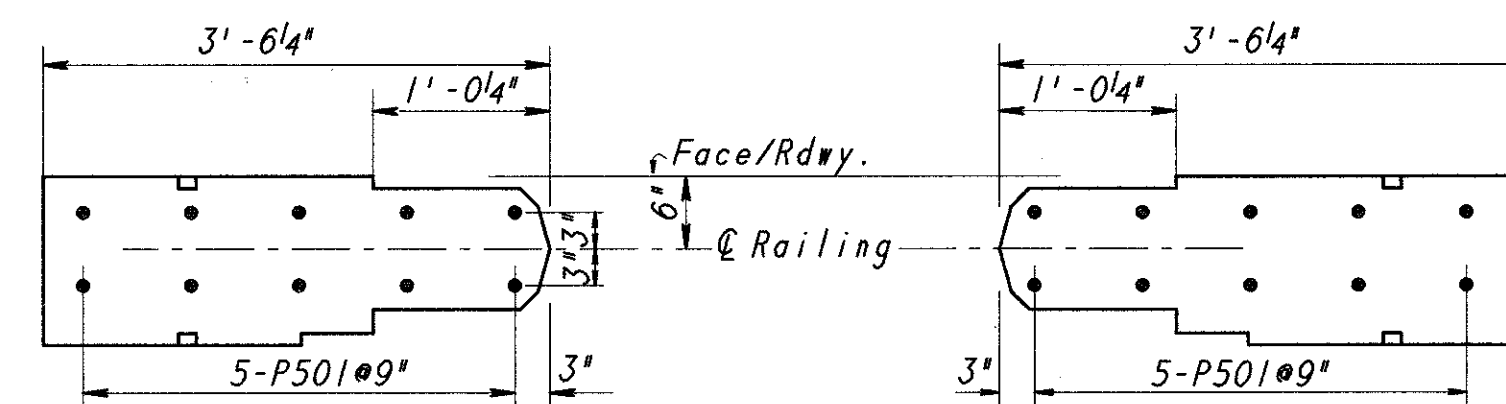
P6L



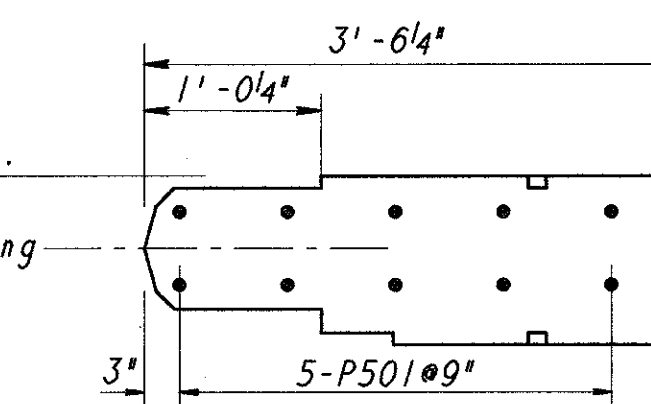
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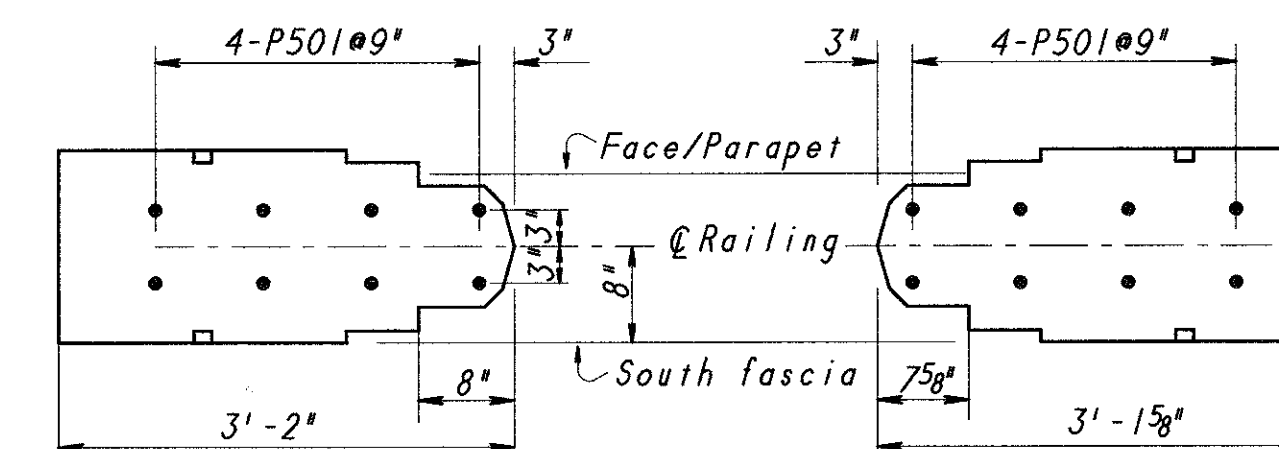
P2M



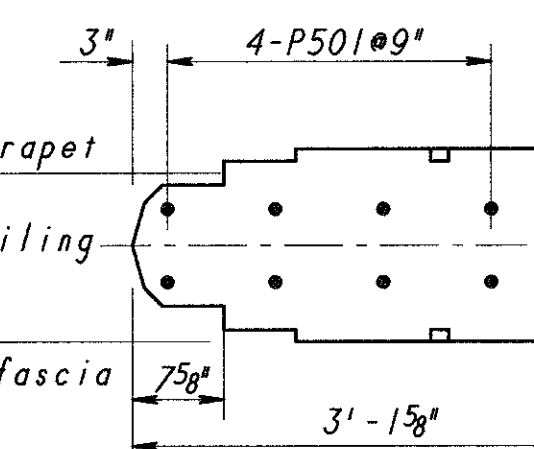
P3M



P4M



P5M



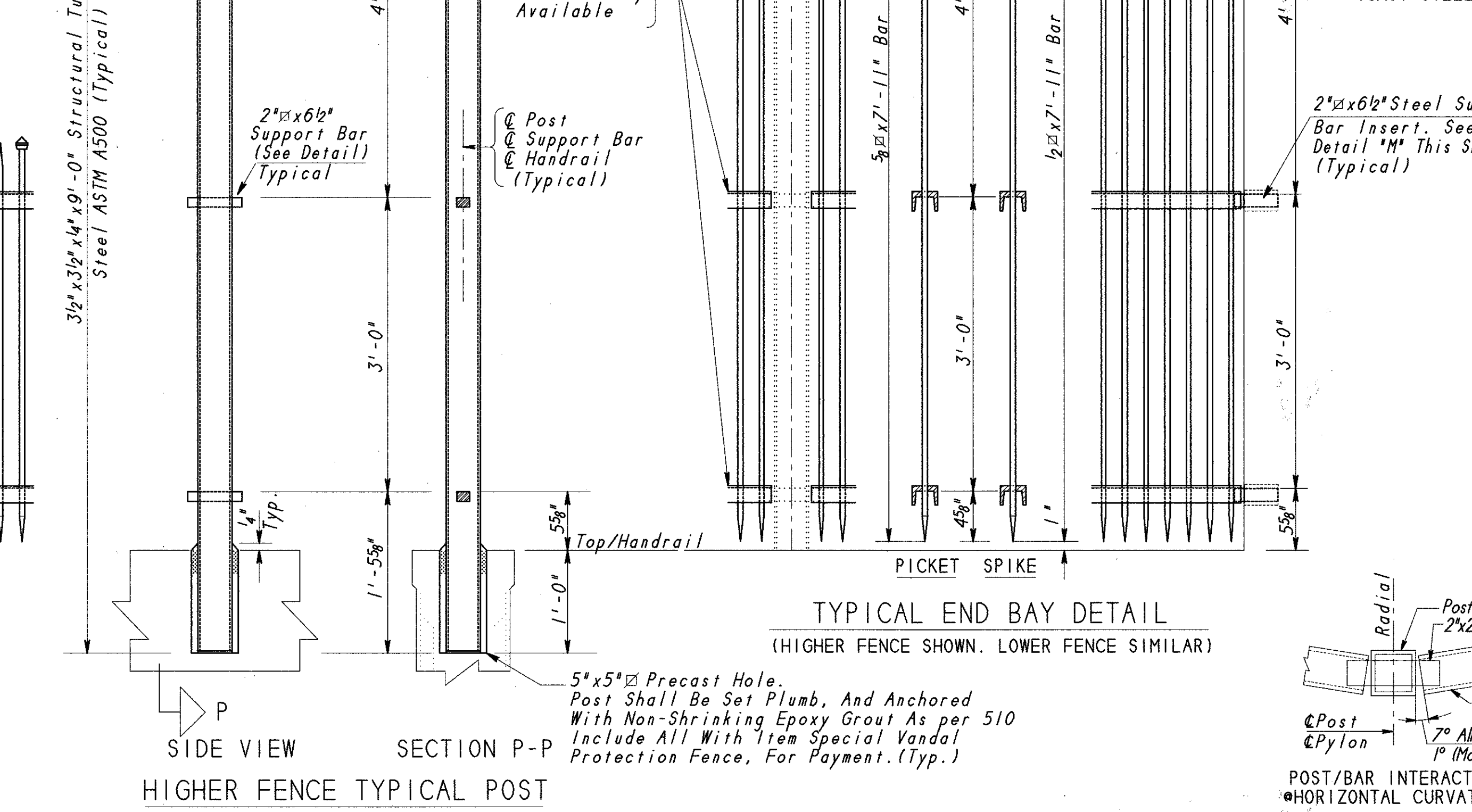
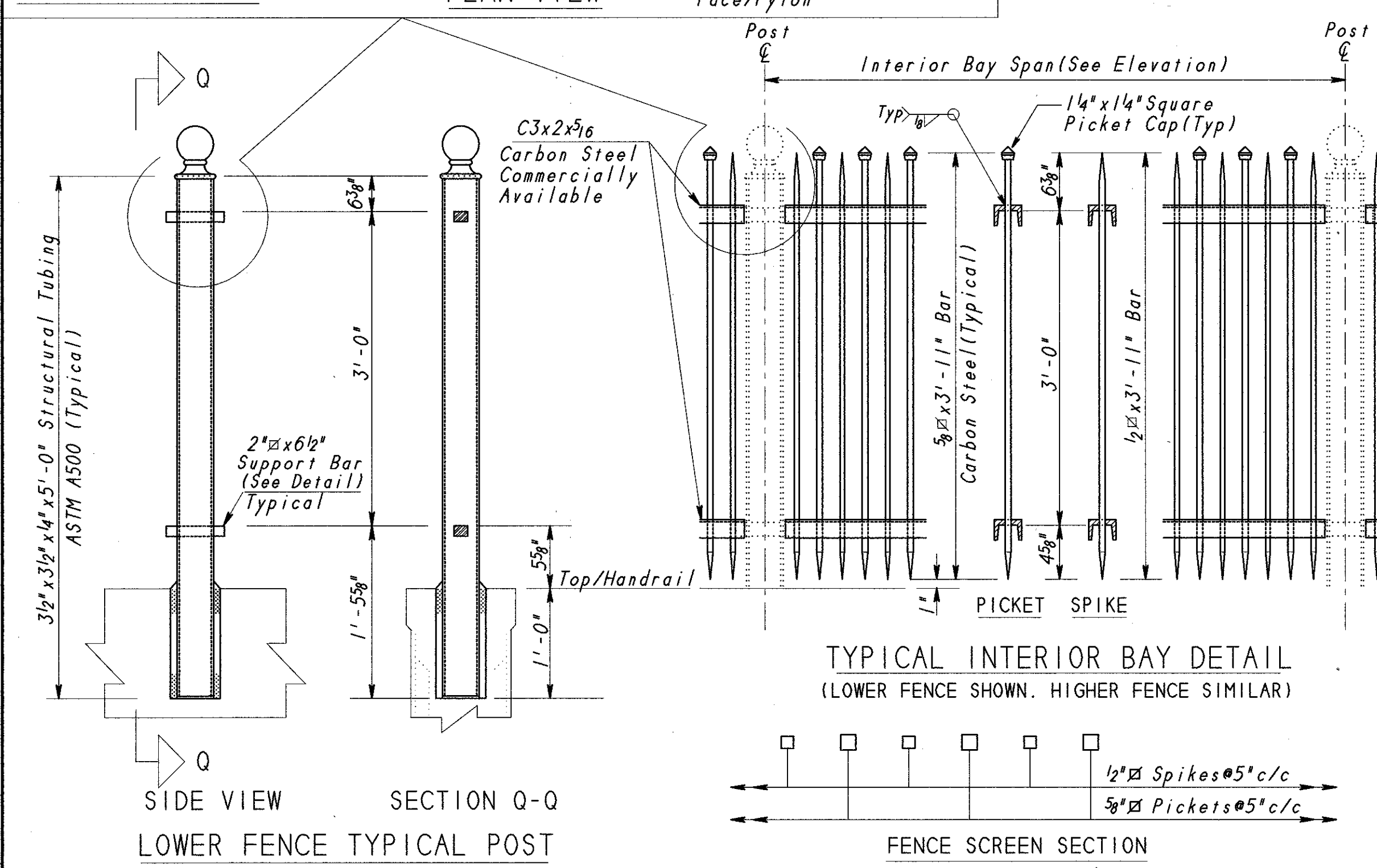
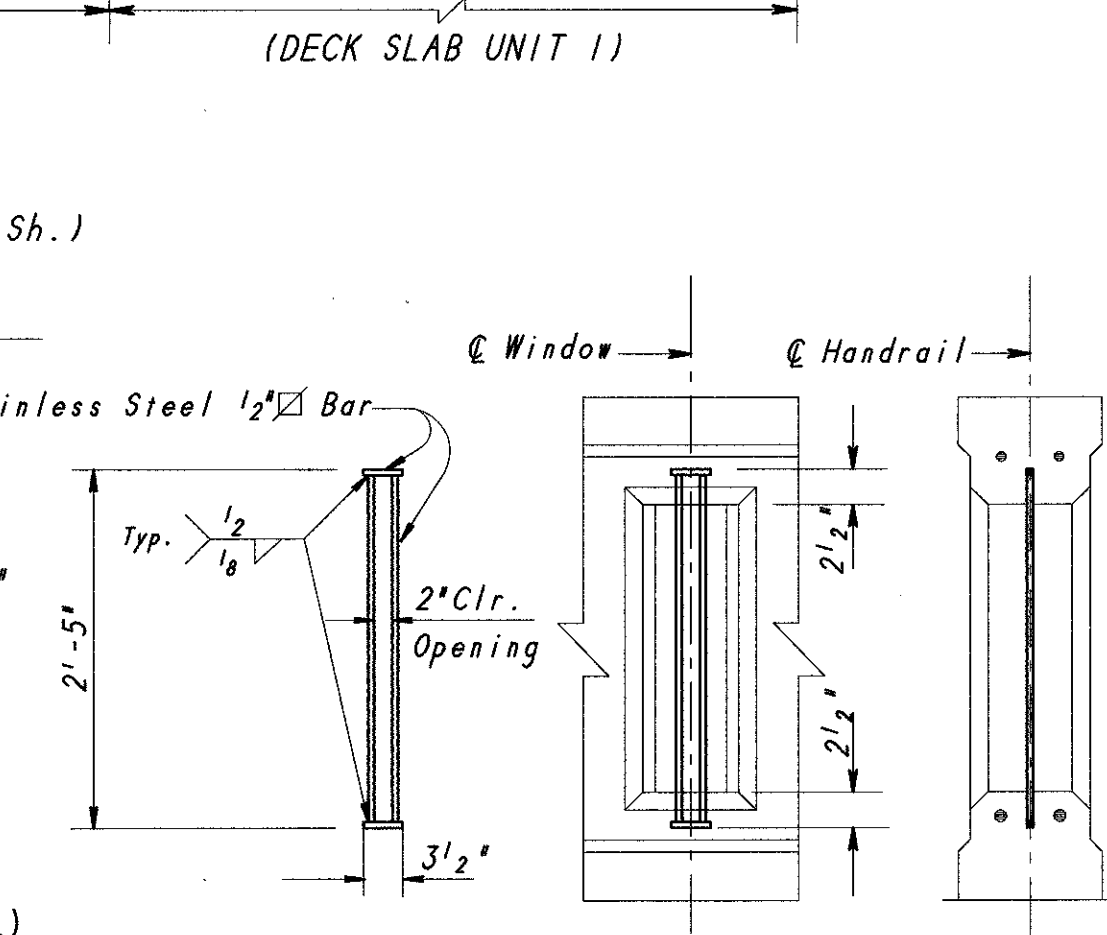
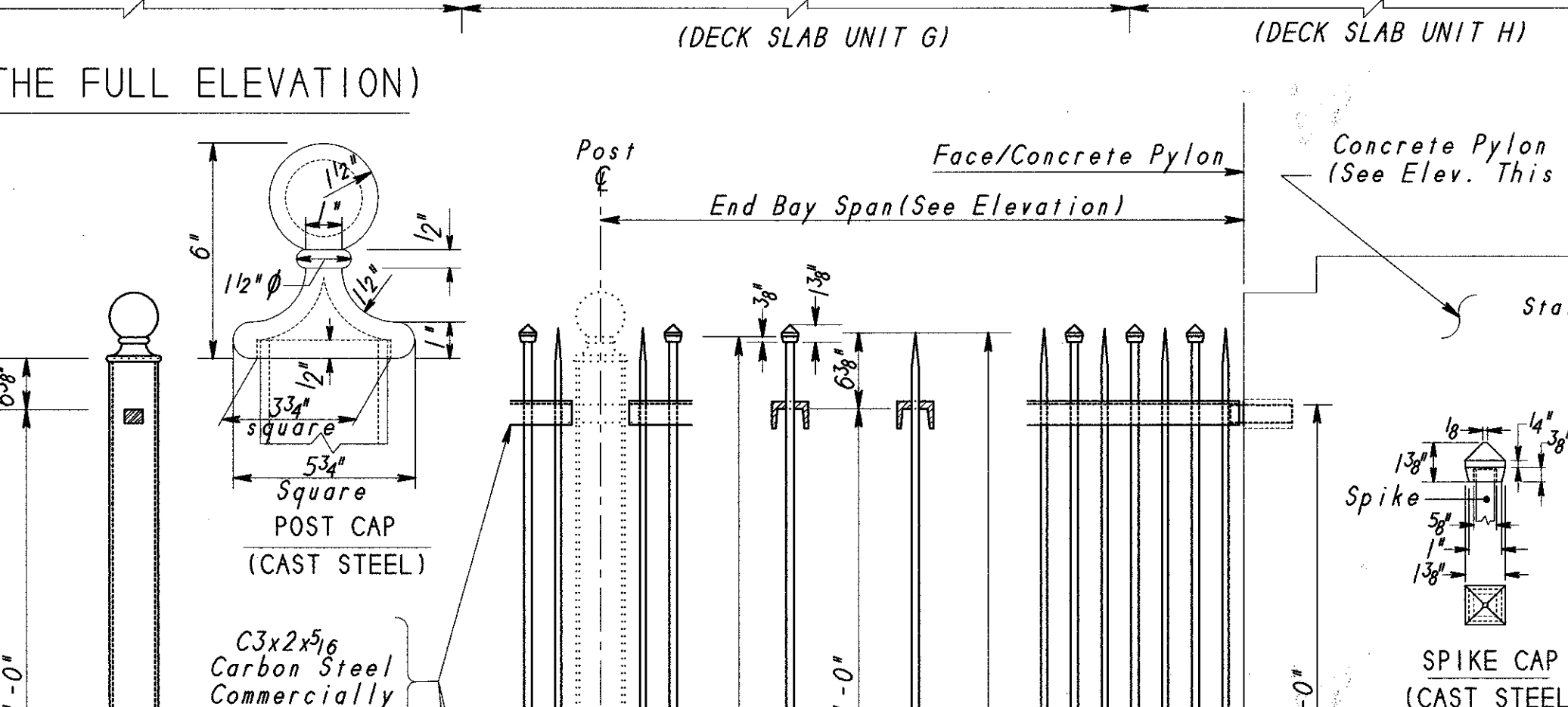
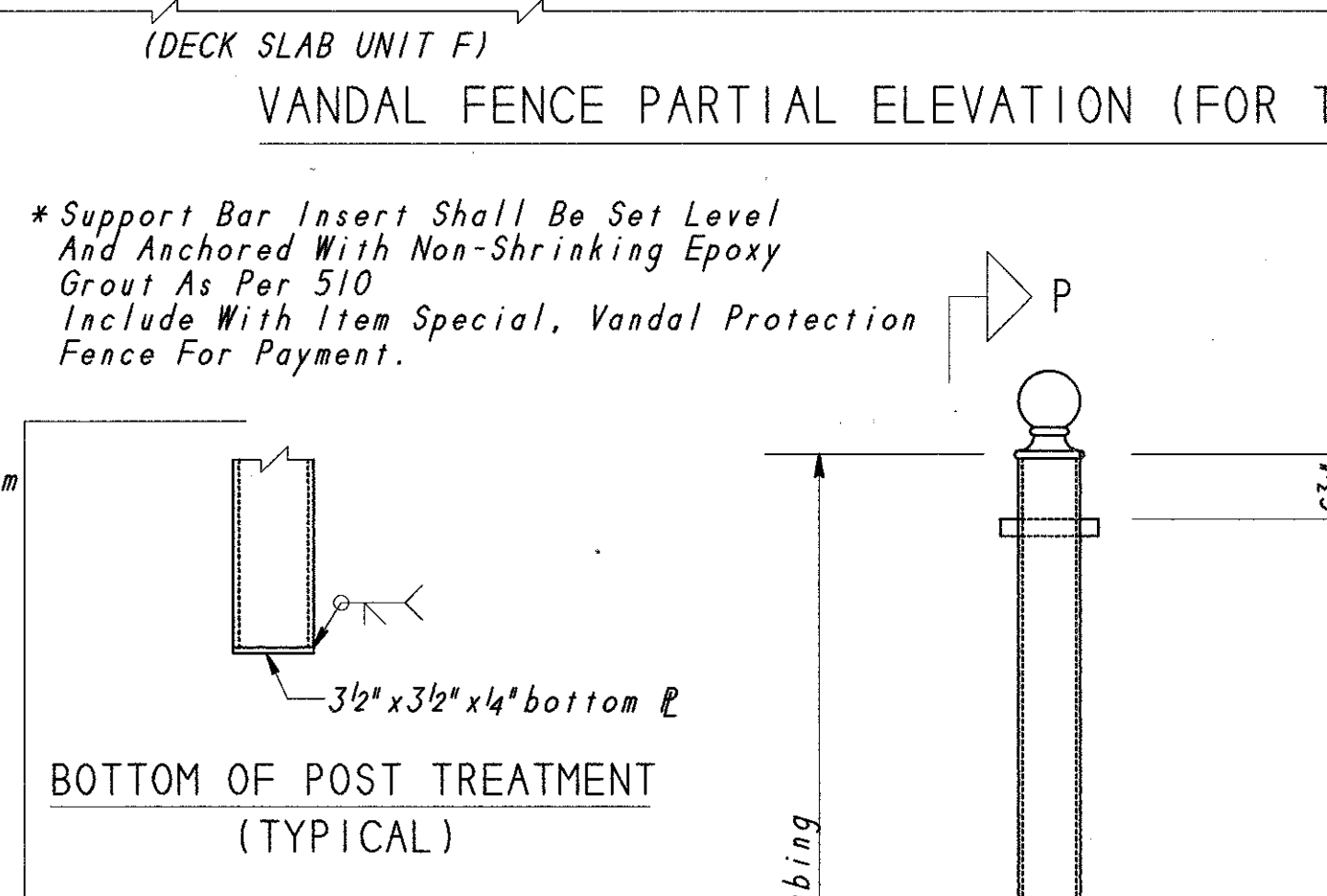
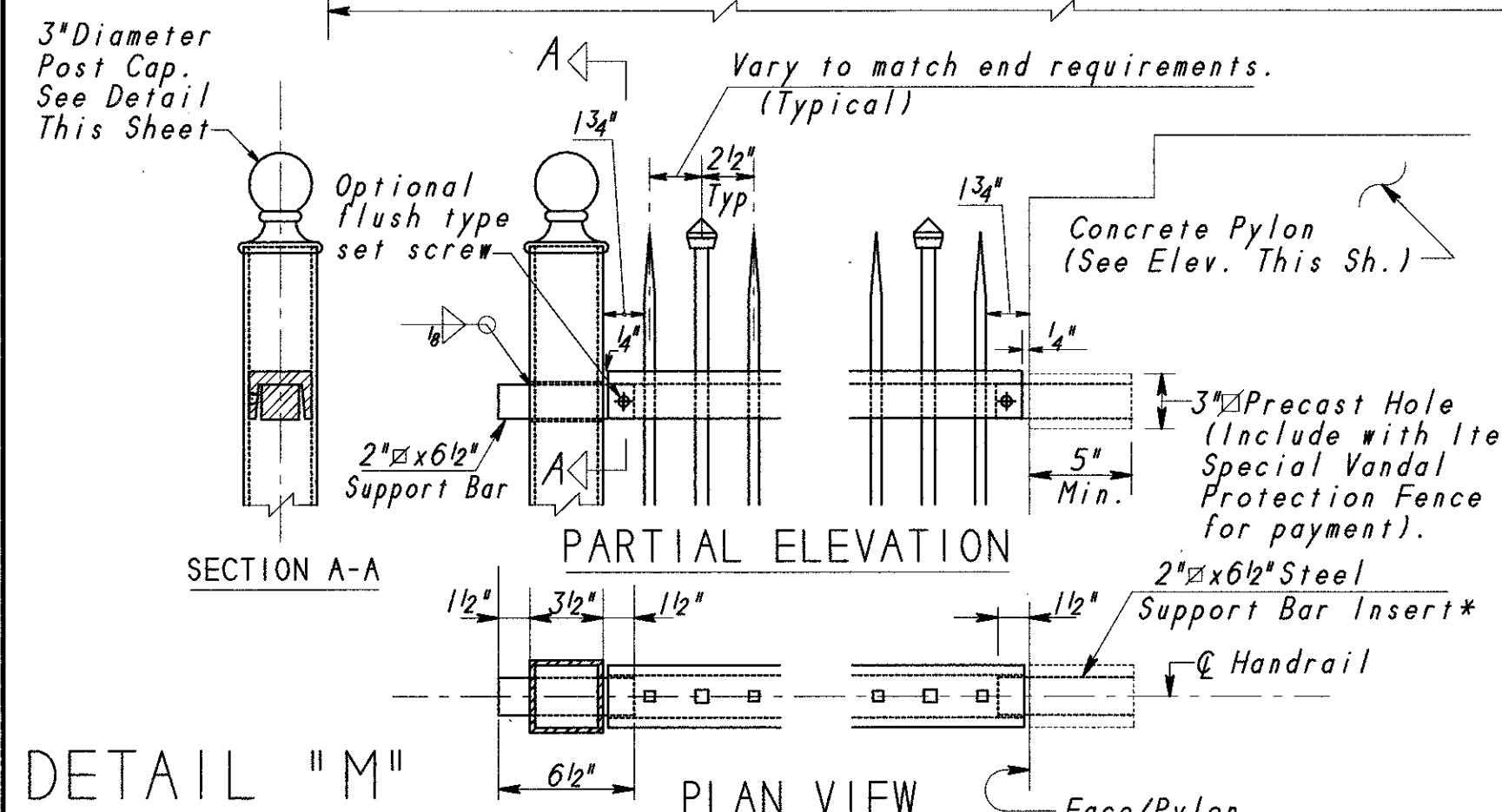
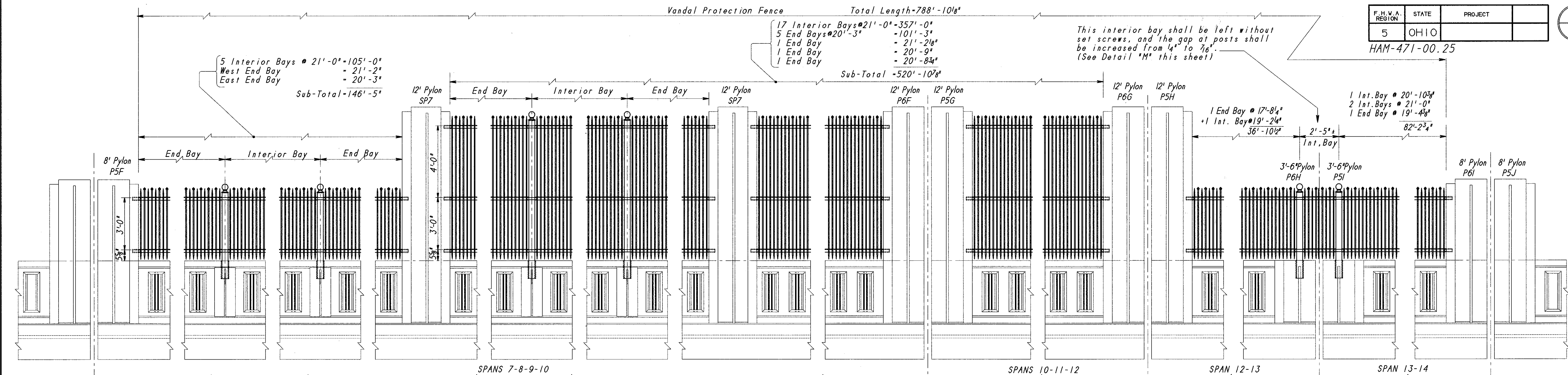
P6M

NOTES:

- 1.-Work this sheet with DECORATIVE RAILING, SPANS 15-16, 16-17, and 17-18 (Slab Units K, L & M), Sh. [80/108]
- 2.-Work this sheet with GENERAL PLAN 1/3 to 3/3 Sh. [4/108], Sh. [5/108] and Sh. [6/108] with DECK SLAB UNIT K, Sh. [71/108] with DECK SLAB UNIT L, Sh. [72/108] with DECK SLAB UNIT M, Sh. [73/108]
- 3.-For additional Notes and Legend, see Sh. [77/108] and Sh. [81/108]
- 4.- See GENERAL NOTES 5/5, Sh. [1/108].

TKEPKLM Scale 1:3333

		84/108
<p>DECORATIVE RAILING END OF SPAN PYLONS (SPANS 15-16, 16-17 & 17-18) (SLAB UNITS K, L, & M) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)</p>		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	CHECKED
ED	ED	WDD
TRACED	DATE	REVIEWED
		IEH April 96



NOTES:

- Fencing and window fence screen to be constructed along South Fascia only.
- See GENERAL NOTES 5/5, Sh. 11108 on Item Special VANDAL PROTECTION FENCE.
- Fence screen at railing windows is included in Unit Price Bid for Item Special, VANDAL PROTECTION FENCE, for payment.
- Screen Spike And Picket Bars Tapering End Detail:

Plum, Klausmeyer & Gehrum 85/108
Consultants OHIO

VANDAL PROTECTION FENCE

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY Sta. 30+55.40
Sta. 47+16.19

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED		WDD	IEH	April 96	

TYWENGE Scale 2:66667

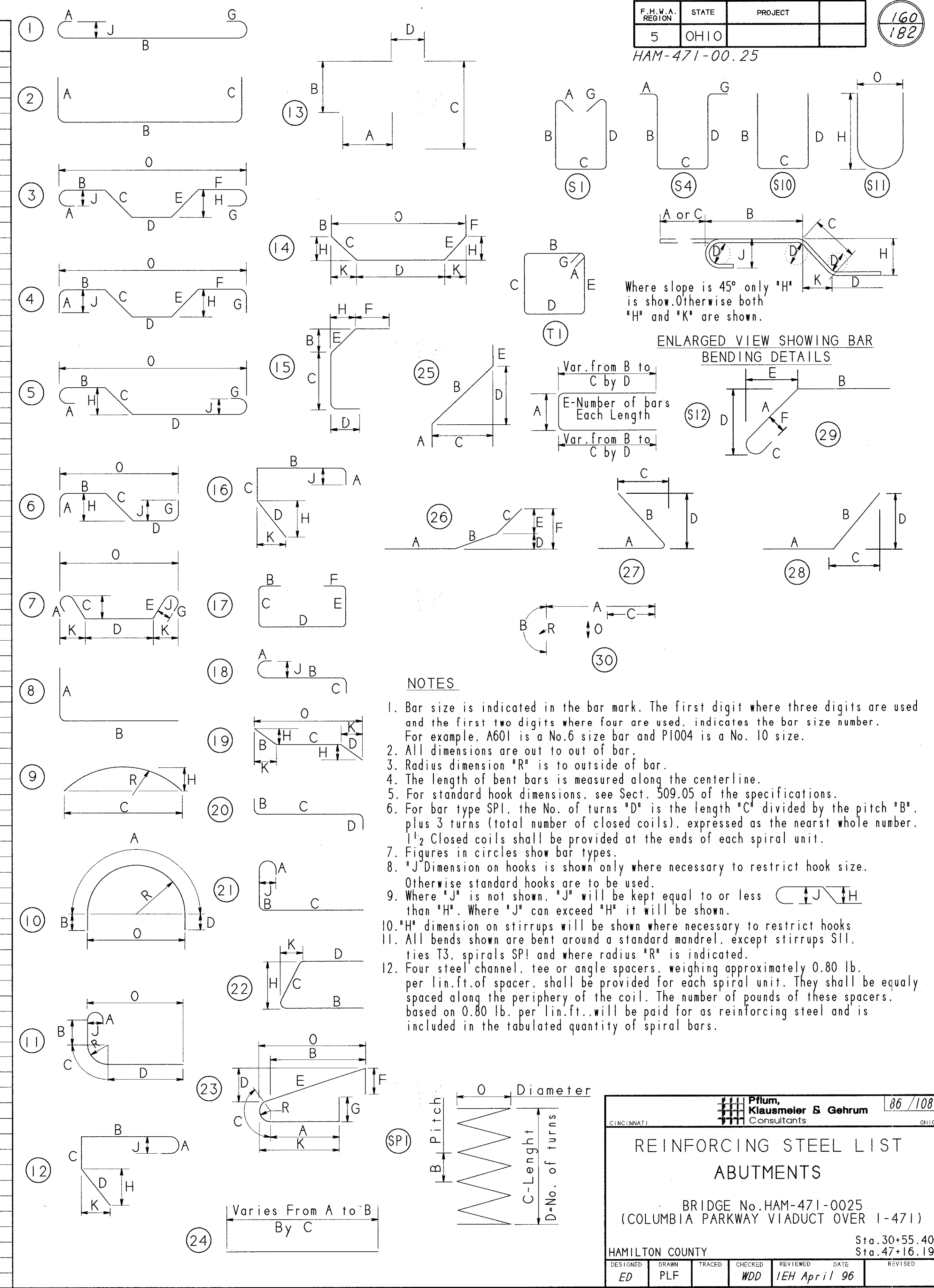
REINFORCING STEEL BAR LIST

F.H.W.A. REGION	STATE	PROJECT
5	OHIO	

160
182

HAM-471-00.25

MARK	No.	LENGTH	WT.	TYPE	A	B	C	D	E	F	G	H	J	K	O	R	NOTES
REAR ABUTMENT (ABUTMENT No. 1)																	
RA401	3	3'-11"	8	T1	0'-4 1/2"	0'-10"	0'-11"	0'-10"	0'-11"		0'-4 1/2"						
RA501	8	32'-0"	267	Str.													
RA502	1	33'-0"	34	Str.													
RA503	8	39'-10"	332	Str.													
RA504	1	31'-0"	32	Str.													
RA505	9	6'-2"	58	Str.													
RA506	3	11'-6"	36	Str.													
RA507	6	7'-8"	48	Str.													
RA508	6	4'-7"	29	8	1'-2"	3'-6 1/2"											Dowels
RA509	5	4'-7"	24	28	1'-2"	3'-6"	2'-5 3/4"	2'-5 3/4"									Dowels
RA601	68	11'-1"	1132	2	5'-0"	1'-5"	5'-0"										
RA602	57	9'-1"	778	2	4'-0"	1'-5"	4'-0"										
RA603	57	6'-5"	549	2	2'-11"	0'-11"	2'-11"										
RA604	11	10'-10"	179	2	4'-10 1/2"	1'-5"	4'-10 1/2"										
RA605	11	5'-9"	95	2	2'-7"	0'-11"	2'-7"										
RA606	4	5'-2"	31	Str.													
RA801	39	5'-5"	564	29	3'-1"	1'-5"	0'-11"	2'-2"	2'-2"	0'-6"							
Sub-Total = 4196 Lb. Epoxy Coated Reinforcing Steel Grade 60																	
FORWARD ABUTMENT (ABUTMENT No. 18)																	
FA401	3	3'-11"	8	T1	0'-4 1/2"	0'-10"	0'-11"	0'-10"	0'-11"		0'-4 1/2"						
FA501	8	31'-6"	263	Str.													
FA502	1	32'-2"	34	Str.													
FA503	8	39'-8"	331	Str.													
FA504	1	31'-0"	32	Str.													
FA505	2	11'-0"	23	Str.													
FA506	1	11'-6"	12	Str.													
FA507	8	8'-0"	67	2	1'-7"	1'-0"	5'-8"										
FA508	6	7'-8"	48	Str.													
FA509	1	5'-8"	6	Str.													
FA601	68	9'-7"	979	2	4'-3"	1'-5"	4'-3"										
FA602	57	9'-1"	778	2	4'-0"	1'-5"	4'-0"										
FA603	57	6'-5"	549	2	2'-11"	0'-11"	2'-11"										
FA604	11	10'-10"	179	2	4'-10 1/2"	1'-5"	4'-10 1/2"										
FA605	11	5'-9"	95	2	2'-7"	0'-11"	2'-7"										
FA606	4	5'-2"	31	Str.													
FA607	8	2'-8"	32	Str.													
FA608	8	3'-6"	42	8	1'-8"	2'-0"											Dowels
FA801	38	5'-5"	550	29	3'-1"	1'-5"	0'-11"	2'-2"	2'-2"	0'-6"							
Sub-Total = 4059 Lb. Epoxy Coated Reinforcing Steel Grade 60																	
TOTAL = 8255 Lb. EPOXY COATED REINFORCING STEEL GRADE 60 - ABUTMENTS.																	



Plum, Klausmeier & Gehrum
Consultants
86/108

REINFORCING STEEL LIST
ABUTMENTS

BRIDGE No. HAM-471-0025
(COLUMBIA PARKWAY VIADUCT OVER I-471)

HAMILTON COUNTY
DESIGNED ED
DRAWN PLF
CHECKED WDD
REVIEWED IEH
DATE April 96
REVISED

Sta. 30+55.40
Sta. 47+16.19

SCALE

REINFORCING STEEL BAR LIST

MARK	No.	LENGTH	WT.	TYPE	A	B	C	D	E	F	G	H	J	K	O	R	NOTES
SC401	1	15'-6"	10	Str.													
SC402	6	1 Series	128	24	26'-0"	37'-10"	2'-4 ¹ / ₂ "										
SC403	11	1 Series	279	24	36'-0"	40'-0"	0'-4 ³ / ₄ "										
SC404	16	1 Series	362	24	32'-0"	35'-9"	0'-3"										
SC405	29	31'-8"	613	Str.													
SC406	9	1 Series	78	24	4'-10"	21'-3"	2'-0 ⁵ / ₈ "										
SC407	9	1 Series	96	24	4'-4"	27'-6"	2'-10 ¹ / ₄ "										
SC408	56	29'-10"	1117	Str.													
SC409	12	1 Series	132	24	4'-4"	28'-6"	2'-2 ³ / ₈ "										
SC410	70	7'-6"	351	1	0'-6"	7'-0"					0		0'-3"				
SC411	14	18'-0"	168	Str.													
SC412	14	40'-0"	374	Str.													
SC413	14	38'-10"	363	Str.													
SC414	86	5'-0"	287	SI	0'-4 ¹ / ₂ "	1'-11"	0'-8"	1'-11"				0'-4 ¹ / ₂ "					
SC501	42	29'-3"	1281	Str.													
SC502	21	40'-0"	876	Str.													
SC503	15	1 Series	381	24	20'-0"	28'-9"	0'-7 ¹ / ₂ "										
SC504	6	1 Series	178	24	27'-9"	29'-0"	0'-3"										
SC505	15	1 Series	448	24	22'-9"	34'-6"	0'-10"										
SC506	5	22'-8"	118	Str.													
SC507	1	14'-6"	15	Str.													
SC508	6	1 Series	200	24	26'-0"	37'-10"	2'-4 ¹ / ₂ "										
SC509	11	1 Series	438	24	36'-3"	40'-0"	0'-4 ¹ / ₂ "										
SC510	16	1 Series	574	24	32'-6"	36'-3"	0'-3"										
SC511	29	32'-3"	975	Str.													
SC512	9	1 Series	127	24	5'-4"	21'-9"	2'-0 ⁵ / ₈ "										
SC513	3	22'-0"	68	28	19'-3"	2'-9"	2'-3"	1'-7"									
SC514	3	27'-3"	85	Str.													
SC515	3	11'-3"	35	Str.													
SC516	3	31'-0"	97	Str.													
SC517	111	4'-7"	531	13	1'-9"	1'-4"	1'-4"	0'-7"									
SC518	21	27'-9"	608	Str.													
SC519	21	36'-8"	803	Str.													
SC520	21	1 Series	572	24	20'-6"	31'-9"	0'-6 ³ / ₄ "										
SC521	21	40'-0"	876	Str.													
SC522	21	1 Series	694	24	23'-4"	40'-0"	0'-10"										
SC523	56	29'-10"	1742	Str.													
SC524	12	1 Series	205	24	4'-4"	28'-6"	2'-2 ³ / ₈ "										
SC525	9	1 Series	154	24	4'-10"	28'-0"	2'-10 ³ / ₄ "										
SC526	71	5'-3"	389	17		0'-10"	1'-4"	1'-5"	1'-4"	0'-10"							
SC527	21	40'-0"	876	Str.													
SC528	21	29'-0"	635	Str.													
SC529	21	40'-0"	876	Str.													
SC530	20	17'-6"	365	Str.													
SC531	3	32'-4"	101	Str.													
SC532	3	36'-10"	115	Str.													
SC533	69	13'-3"	954	1	0'-7"	12'-8"			0			0'-3 ³ / ₄ "					
SC534	70	12'-7"	919	1	0'-7"	12'-0"			0			0'-3 ³ / ₄ "					
SC535	1	13'-2"	14	28	10'-8"	2'-6"	2'-0"	1'-6"									
SC536	124	6'-0"	776	13	1'-9"	1'-1"	2'-10"	0'-7"									

REINFORCING STEEL BAR LIST

MARK	No.	LENGTH	WT.	TYPE	A	B	C	D	E	F	G	H	J	K	O	R	NOTES
SC601	21	40'-0"	1262	Str.													
SC602	16	1 Series	777	24	27'-6"	37'-2"	0'-7 ³ / ₄ "										
SC603	5	1 Series	272	24	35'-9"	36'-9"	0'-3"										
SC604	15	1 Series	414	24	14'-0"	22'-9"	0'-7 ¹ / ₂ "										
SC605	6	1 Series	207	24	22'-4"	23'-6"	0'-3"										
SC606	8	35'-6"	427	Str.													
SC607	16	1 Series	296	24	7'-6"	17'-2"	0'-7 ³ / ₄ "										
SC608	5	1 Series	122	24	15'-9"	16'-9"	0'-3"										
SC609	16	1 Series	529	24	15'-6"	28'-6"	0'-10 ³ / ₈ "										
SC610	16	1 Series	390	24	10'-0"	22'-6"	0'-10"										
SC611	15	1 Series	470	24	15'-0"	26'-9"	0'-10"										
SC612	6	11'-0"	99	Bent													Bent on Site
SC613	12	40'-0"	721	Bent													Bent on Site
SC614	2	16'-0"	48	Str.													
SC615	5	18'-2"	136	Str.													
SC616	5	16'-0"	120	Str.													
SC617	2	23'-0"	69	Str.													
SC618	4	16'-3"	98	Bent													Bent on Site
SC619	8	40'-0"	481	Bent													Bent on Site
SC620	20	1 Series	1035	24	28'-11"	40'-0"	0'-7"										
SC621	21	40'-0"	1262	Str.													
SC622	21	1 Series	777	24	16'-3"	33'-0"	0'-10"										
SC623	21	1 Series	678	24	15'-3"	27'-9"	0'-7 ¹ / ₂ "										
SC624	22	1 Series	540	24	7'-6"	25'-2"	0'-10"										
SC625	20	1 Series	807	24	21'-3"	32'-6"	0'-7 ¹ / ₈ "										
SC626	21	1 Series	568	24	9'-8"	26'-4"	0'-10"										
SC627	8	22'-0"	264	Bent													Bent on Site
SC628	16	40'-0"	961	Bent													Bent on Site
SC629	6	20'-3"	183	Bent													Bent on Site
SC630	12	40'-0"	721	Bent													Bent on Site
SC631	4	15'-0"	90	Str.													
SC632	20	2 Series of 10	544	24	6'-9"	29'-6"	2'-6"										
SC633	5	10'-3"	77	Str.													
Sub-Total = 37,904 Lbs. Epoxy Coated Reinforcing Steel Grade 60																	

NOTES:

1. Bar size is indicated in the bar mark. The first digit where three digits are used and the first two digits where four are used, indicate the bar size numbers. For example, A601 is a No. 6 size bar and P1004 is a No. 10 size.
2. For bar types see sheet. [86/108]
3. All dimensions are out to out of bar.
4. Radius dimension "R" is to outside of bar.
5. The length of bent bars is measured along the centerline.
6. For additional notes see sheet. [86/108]

Pflum, Klausmeyer & Gehrmann CONSULTANTS				90/108
REINFORCING STEEL LIST				
DECK SPAN 3-4				
(UNIT C)				
BRIDGE No. HAM-471-0025				
(COLUMBIA PARKWAY VIADUCT OVER I-471)				
HAMILTON COUNTY				Sta. 30+55.40
				Sta. 47+16.19
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
ED	JDR	WDD	IEH	April 96

REINFORCING STEEL BAR LIST

Table with columns: MARK, No., LENGTH, WT., TYPE, A, B, C, D, E, F, G, H, J, K, O, R, NOTES. Rows include SG401-412 and SG501-555.

REINFORCING STEEL BAR LIST

Table with columns: MARK, No., LENGTH, WT., TYPE, A, B, C, D, E, F, G, H, J, K, O, R, NOTES. Rows include SG556-559 and SG601-618.

Sub-Total = 76,227 Lbs. Epoxy Coated Reinforcing Steel Grade 60

NOTES:

- 1. Bar size is indicated in the bar mark. The first digit where three digits are used...
2. For bar types see sheet. 86/108
3. All dimensions are out to out of bar.
4. Radius dimension "R" is to outside of bar.
5. The length of bent bars is measured along the centerline.
6. For additional notes see sheet. 86/108

REINFORCING STEEL LIST
DECK SPAN 10-11-12
(BRIDGE No. HAM-471-0025)
(COLUMBIA PARKWAY VIADUCT OVER I-471)
HAMILTON COUNTY
DESIGNED ED DRAWN JDR TRACED WDD CHECKED WDD REVIEWED IEH DATE April 96 REVISIONS

UNIT UNITS

DECK SPAN 13-14 (UNIT "I")

REINFORCING STEEL BAR LIST

MARK	No.	LENGTH	WT.	TYPE	A	B	C	D	E	F	G	H	J	K	O	R	NOTES
S1401	60	31'-6"	1263	Str.													
S1402	62	30'-0"	1242	Str.													
S1403	28	24'-9"	463	Str.													
S1404	14	40'-0"	374	Str.													
S1405	64	7'-6"	321	I	0'-6"	7'-0"					0		0'-3"				
S1406	80	5'-0"	267	SI	0'-4 1/2"	1'-11"	0'-8"	1'-11"			0'-4 1/2"						
S1501	20	40'-0"	834	Str.													
S1502	20	26'-0"	542	Str.													
S1503	21	40'-0"	876	Str.													
S1504	21	24'-0"	526	Str.													
S1505	21	1 Series	492	24	21'-2"	23'-9"	0'-1 1/2"										
S1506	20	26'-0"	542	Str.													
S1507	20	34'-6"	720	Str.													
S1508	20	1 Series	542	24	24'-9"	27'-3"	0'-1 5/8"										
S1509	21	35'-3"	772	Str.													
S1510	21	35'-0"	767	Str.													
S1511	21	1 Series	358	24	15'-0"	17'-8"	0'-1 5/8"										
S1512	4	20'-0"	83	Str.													
S1513	62	31'-11"	2064	Str.													
S1514	102	4'-7"	488	13	1'-9"	1'-4"	1'-4"	0'-7"									
S1515	20	33'-6"	699	Str.													
S1516	20	32'-0"	668	Str.													
S1517	21	24'-9"	542	Str.													
S1518	21	40'-0"	876	Str.													
S1519	21	1 Series	543	24	23'-6"	26'-1"	0'-1 1/2"										
S1520	20	28'-6"	595	Str.													
S1521	20	33'-6"	699	Str.													
S1522	20	1 Series	571	24	26'-3"	28'-6"	0'-1 3/8"										
S1523	21	37'-8"	825	Str.													
S1524	21	35'-0"	767	Str.													
S1525	21	1 Series	378	24	16'-0"	18'-6"	0'-1 1/2"										
S1526	4	21'-2"	88	Str.													
S1527	64	30'-0"	2003	Str.													
S1528	64	5'-3"	350	17		0'-10"	1'-4"	1'-5"	1'-4"	0'-10"							
S1529	63	13'-3"	871	I	0'-7"	12'-8"					0		0'-3 3/4"				
S1530	63	12'-7"	827	I	0'-7"	12'-0"					0		0'-3 3/4"				
S1531	2	12'-8"	26	Str.													
S1532	111	6'-0"	695	13	1'-9"	1'-1"	2'-10"	0'-7"									
S1533	4	8'-2"	34	13	1'-9"	3'-1"	3'-1"	0'-7"									
S1534	4	9'-6"	40	13	1'-9"	2'-10"	4'-8"	0'-7"									
S1601	20	12'-8"	381	Str.													
S1602	20	1 Series	366	24	11'-0"	13'-4"	0'-1 1/2"										
S1603	6	23'-9"	214	Bent													Bent on Site
S1604	10	40'-0"	601	Bent													Bent on Site
S1605	6	23'-9"	214	Bent													Bent on Site
S1606	4	24'-0"	144	Bent													Bent on Site
S1607	4	25'-3"	152	Bent													Bent on Site
S1608	20	14'-4"	431	Str.													
S1609	20	1 Series	428	24	13'-0"	15'-6"	0'-1 1/2"										
S1610	8	27'-0"	324	Bent													Bent on Site
S1611	8	40'-0"	481	Bent													Bent on Site
S1612	8	25'-0"	300	Bent													Bent on Site
S1613	6	27'-3"	246	Bent													Bent on Site
S1614	6	40'-0"	360	Bent													Bent on Site
S1615	6	26'-0"	234	Bent													Bent on Site
Sub-Total = 30,500 Lbs. Epoxy Coated Reinforcing Steel Grade 60																	

UNIT I/J Scale 1

DECK SPAN 14-15 (UNIT "J")

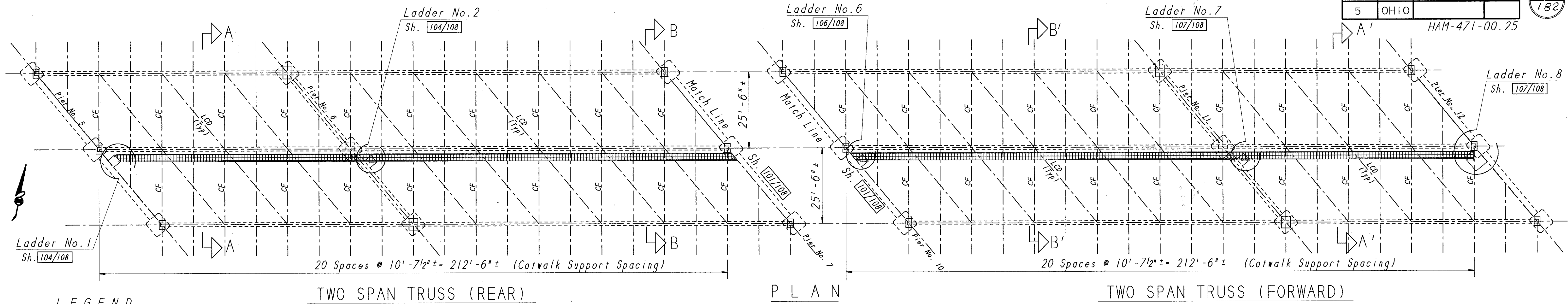
REINFORCING STEEL BAR LIST

MARK	No.	LENGTH	WT.	TYPE	A	B	C	D	E	F	G	H	J	K	O	R	NOTES
SJ401	60	31'-6"	1263	Str.													
SJ402	62	30'-0"	1242	Str.													
SJ403	28	24'-9"	463	Str.													
SJ404	14	40'-0"	374	Str.													
SJ405	64	7'-6"	321	I	0'-6"	7'-0"					0		0'-3"				
SJ406	80	5'-0"	267	SI	0'-4 1/2"	1'-11"	0'-8"	1'-11"			0'-4 1/2"						
SJ501	20	40'-0"	834	Str.													
SJ502	20	26'-0"	542	Str.													
SJ503	21	40'-0"	876	Str.													
SJ504	21	24'-0"	526	Str.													
SJ505	21	1 Series	492	24	21'-2"	23'-9"	0'-1 1/2"										
SJ506	20	26'-0"	542	Str.													
SJ507	20	34'-6"	720	Str.													
SJ508	20	1 Series	542	24	24'-9"	27'-3"	0'-1 5/8"										
SJ509	21	35'-3"	772	Str.													
SJ510	21	35'-0"	767	Str.													
SJ511	21	1 Series	358	24	15'-0"	17'-8"	0'-1 5/8"										
SJ512	4	20'-0"	83	Str.													
SJ513	62	31'-11"	2064	Str.													
SJ514	102	4'-7"	488	13	1'-9"	1'-4"	1'-4"	0'-7"									
SJ515	20	33'-6"	699	Str.													
SJ516	20	32'-0"	668	Str.													
SJ517	21	24'-9"	542	Str.													
SJ518	21	40'-0"	876	Str.													
SJ519	21	1 Series	543	24	23'-6"	26'-1"	0'-1 1/2"										
SJ520	20	28'-6"	595	Str.													
SJ521	20	33'-6"	699	Str.													
SJ522	20	1 Series	571	24	26'-3"	28'-6"	0'-1 3/8"										
SJ523	21	37'-8"	825	Str.													
SJ524	21	35'-0"	767	Str.													
SJ525	21	1 Series	378	24	16'-0"	18'-6"	0'-1 1/2"										
SJ526	4	21'-2"	88	Str.													
SJ527	64	30'-0"	2003	Str.													
SJ528	64	5'-3"	350	17		0'-10"	1'-4"	1'-5"	1'-4"	0'-10"							
SJ529	63	13'-3"	871	I	0'-7"	12'-8"					0		0'-3 3/4"				
SJ530	63	12'-7"	827	I	0'-7"	12'-0"					0		0'-3 3/4"				
SJ531	2	12'-8"	26	Str.													
SJ532	111	6'-0"	695	13	1'-9"	1'-1"	2'-10"	0'-7"									
SJ533	4	8'-2"	34	13	1'-9"	3'-1"	3'-1"	0'-7"									
SJ534	4	9'-6"	40	13	1'-9"	2'-10"	4'-8"	0'-7"									
SJ601	20	12'-8"	381	Str.													
SJ602	20	1 Series	366	24	11'-0"	13'-4"	0'-1 1/2"										
SJ603	6	22'-9"	205	Bent													Bent on Site
SJ604	10	40'-0"	601	Bent													Bent on Site
SJ605	6	23'-9"	214	Bent													Bent on Site
SJ606	4	24'-0"	144	Bent													Bent on Site
SJ607	4	25'-3"	152	Bent													Bent on Site
SJ608	20	14'-4"	431	Str.													
SJ609	20	1 Series	428	24	13'-0"	15'-6"	0'-1 1/2"										
SJ610	8	27'-0"	324	Bent													Bent on Site
SJ611	8	40'-0"	481	Bent													Bent on Site
SJ612	8	25'-0"	300	Bent													Bent on Site
SJ613	6	27'-3"	246	Bent													Bent on Site
SJ614	6	40'-0"	360	Bent													Bent on Site
SJ615	6	26'-0"	234	Bent													Bent on Site
Sub-Total = 30,500 Lbs. Epoxy Coated Reinforcing Steel Grade 60																	

Sub-Total = 30,500 Lbs. Epoxy Coated Reinforcing Steel Grade 60

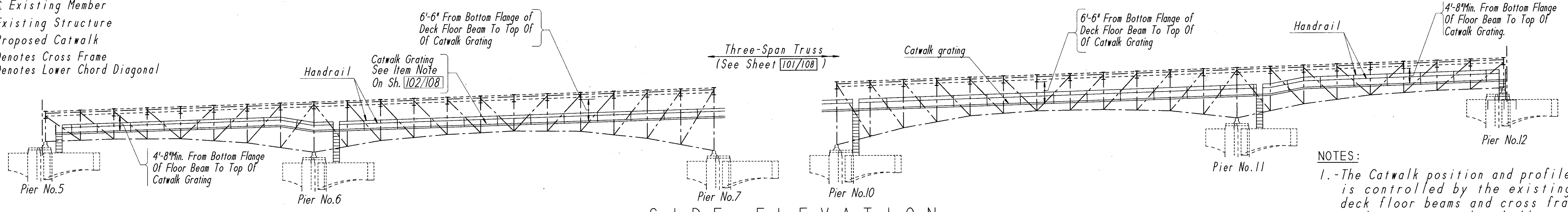
NOTES:

1. Bar size is indicated in the bar mark. The first digit where three digits are used and the first two digits where four are used, indicate the bar size numbers. For example, A601 is a No. 6 size bar and P1004 is a No. 10 size.
2. For bar types see sheet. 86/108
3. All dimensions are out to out of bar.
4. Radius dimension "R" is to outside of bar.
- 5.



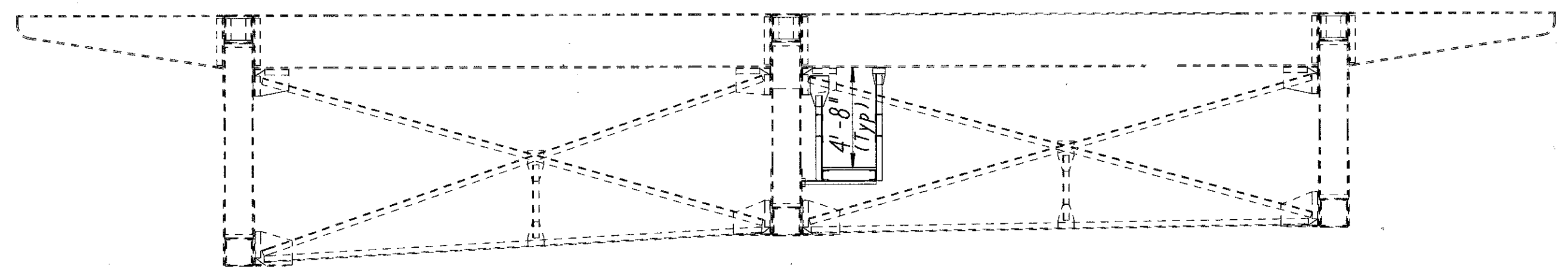
LEGEND

- Existing Member
- - - Existing Structure
- ▨ Proposed Catwalk
- CF Denotes Cross Frame
- LCD Denotes Lower Chord Diagonal

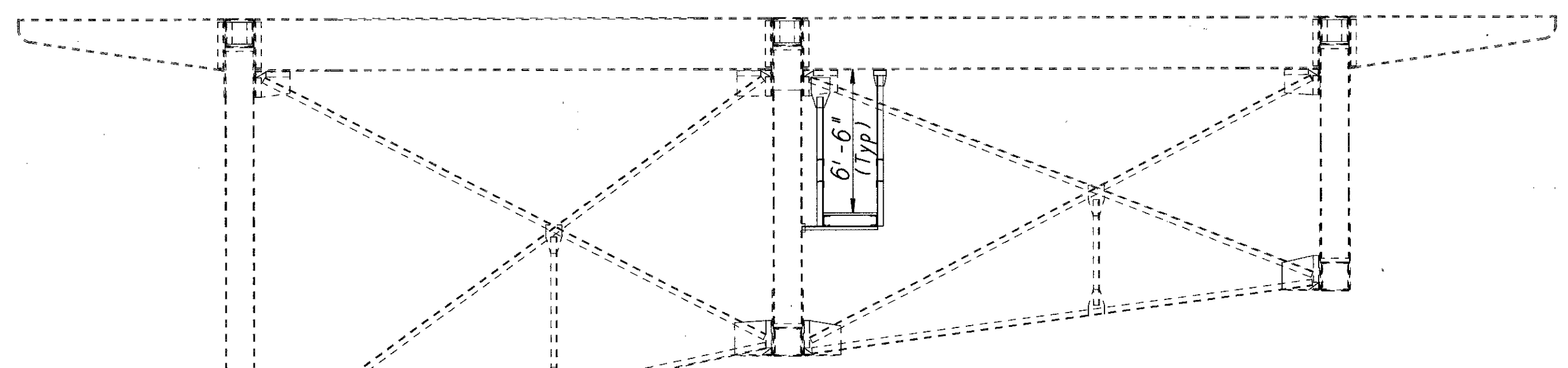


SIDE ELEVATION
(Looking North)

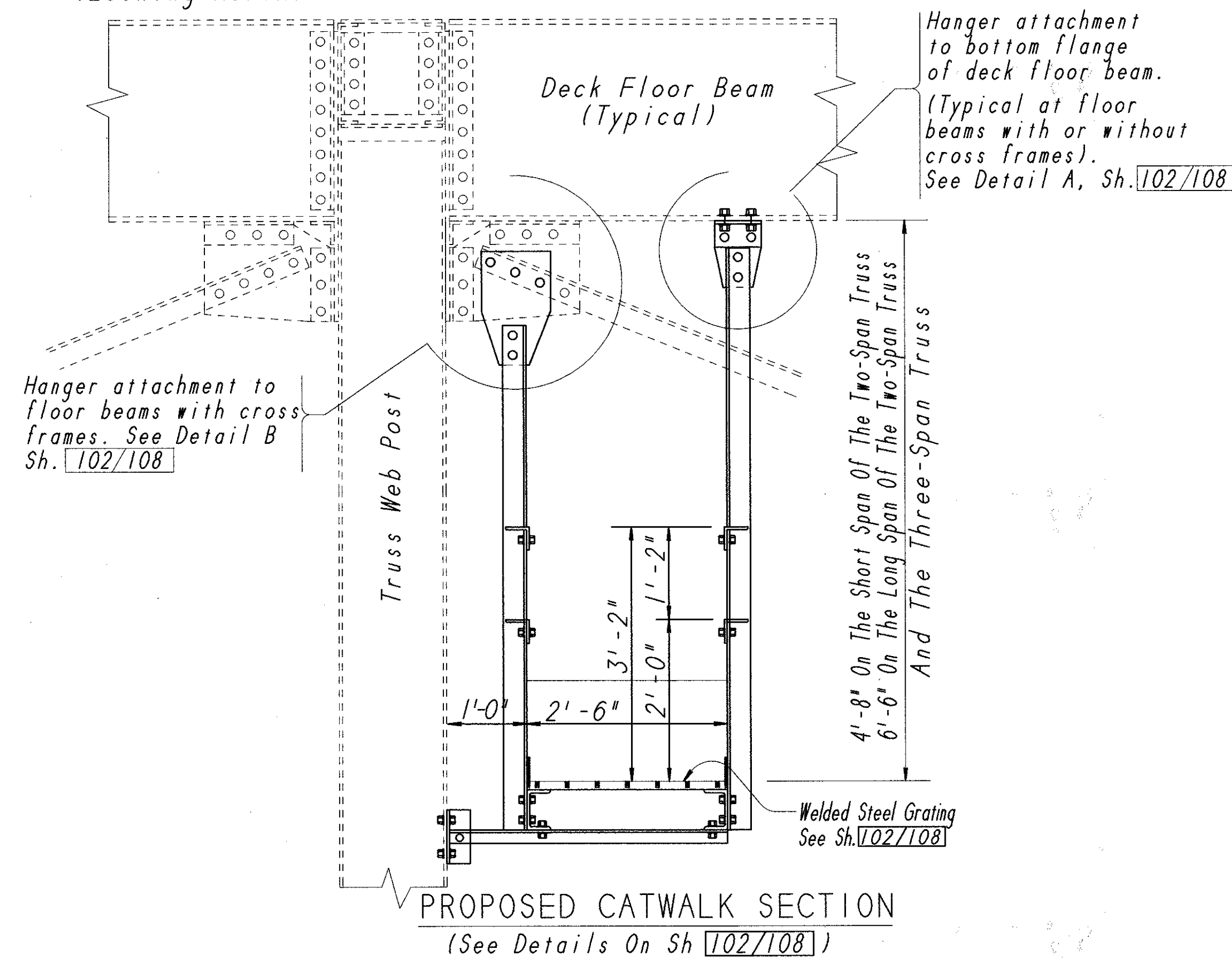
- NOTES:**
- 1.-The Catwalk position and profile is controlled by the existing deck floor beams and cross frame combination at the shallowest depth of the truss.
 - 2.-Representation of the existing structure is approximate.
 - 3.-Work this sheet with sh. 101/108



SECTION A-A
(SECTION A'-A' IS SIMILAR)



SECTION B-B
(SECTION B'-B' IS SIMILAR)



PROPOSED CATWALK SECTION
(See Details On Sh 102/108)

PROPOSED CATWALK STRUCTURE

TYPE: Rolled Sections Of Structural Steel On Bolted Connections Supported On Existing Steel Structure.

SPANS: Multi-Span @ 10'-7 1/2" ± Center To Center Of Supports.

WIDTH: 2'-6" Face To Face Of Handrails

WEARING SURFACE: Welded Steel Grating

LOADING: 85 Pounds Per Square Foot.

CONSULTANTS: Pflum, Klausmeier & Gehrum
CINCINNATI OHIO 100/108

CATWALK PLAN AND ELEVATION 1/2

BRIDGE No. HAM-471-0025
(Columbia Parkway Viaduct Over I-471)

HAMILTON COUNTY
Sta. 30+55.40
Sta. 47+16.19

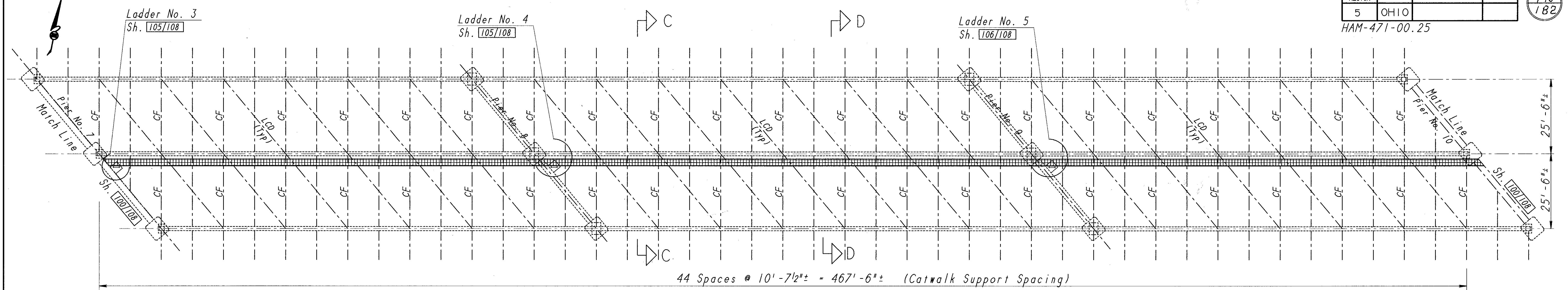
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
ED	ED		WDD	IEH	April 96	

WALKY Scale 1/6

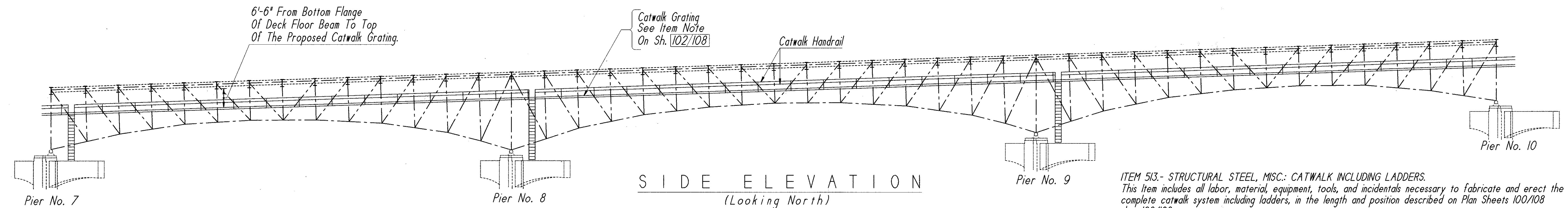
F.H.W.A. REGION	STATE	PROJECT
5	OHIO	

175
182

HAM-471-00.25



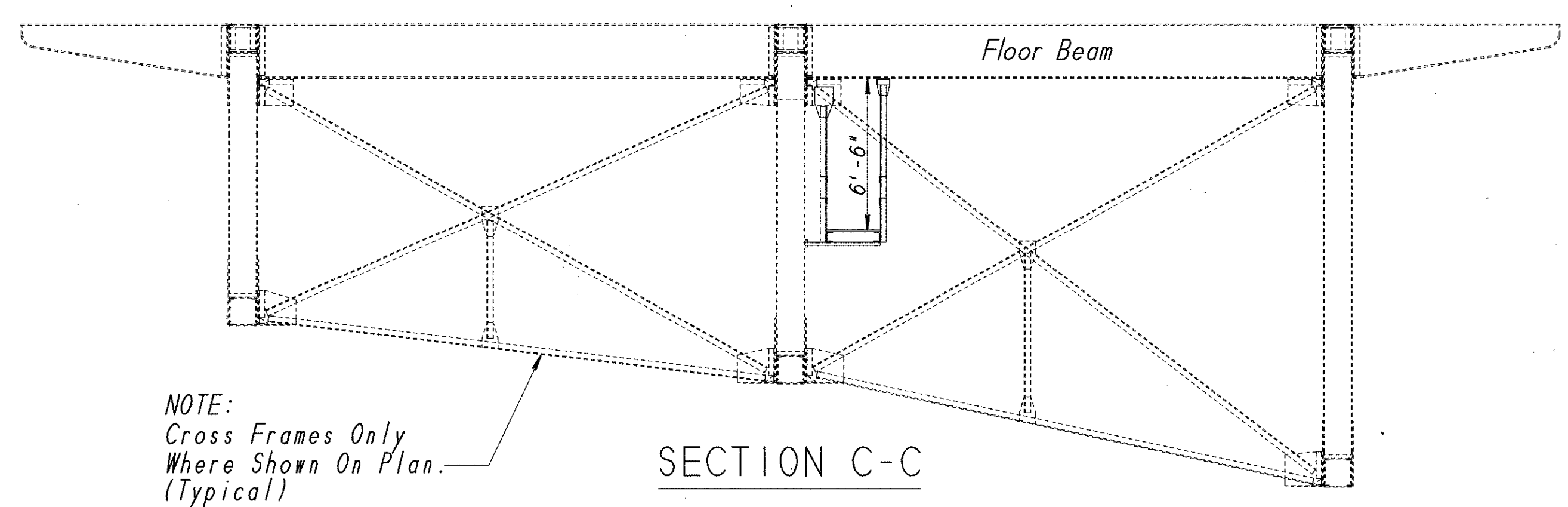
PLAN



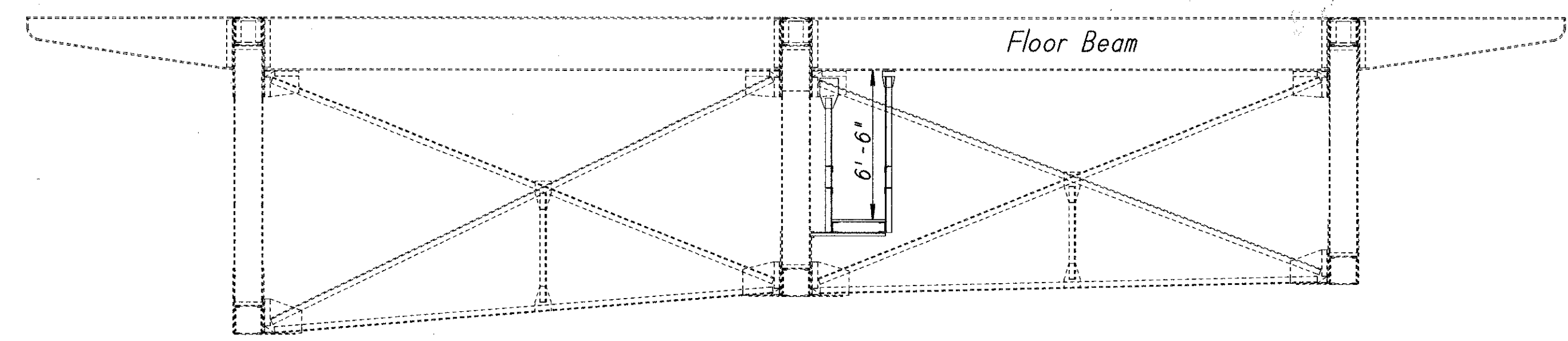
SIDE ELEVATION
(Looking North)

ITEM 513- STRUCTURAL STEEL, MISC.: CATWALK INCLUDING LADDERS.
 This item includes all labor, material, equipment, tools, and incidentals necessary to fabricate and erect the complete catwalk system including ladders, in the length and position described on Plan Sheets 100/108 thru 108/108.
 Prior to fabrication, the Contractor shall submit detailed shop drawings to the Engineer for review and approval. After installation, the Contractor shall submit as-built drawings to the Engineer to ensure that the drawings depict the steel as actually incorporated into the work. Pay weights will be computed in accordance with CMS513. The Contractor shall furnish a 35mm. microfilm copy of each shop drawing, which shall be mounted on an aperture card as per CMS501.05. Steel members described in this item include longitudinal channels, main hangers (including drilling of the existing steel members designated to receive the hangers), railing, ladders, and ladder landings (including doweled anchor bolts, etc.), and ladder cages.
PAINTING: Painting of the catwalk including ladders, ladder cages and landings shall conform to Item 514 System IZEU, retouched after erection (See Proposal Note). Color of paint shall be black, or as approved by the City of Cincinnati. Painting is included with Item 514, PAINTING OF NEW STEEL, SYSTEM IZEU, for payment.
METHOD OF PAYMENT: The pay quantity for catwalk (including ladders, ladder cages, and landings) will be based on the weight in pounds of the structure installed, inspected, approved, and accepted by the Engineer.
BASIS OF PAYMENT: Payment will be made at the Contract unit price bid per pound for Item 513, STRUCTURAL STEEL MISC.: CATWALK INCLUDING LADDERS.

- LEGEND**
- Existing Member
 - Existing Structure
 - Proposed Catwalk
 - CF Denotes Cross Frame
 - LCD Denotes Lower Chord Diagonal



SECTION C-C



SECTION D-D

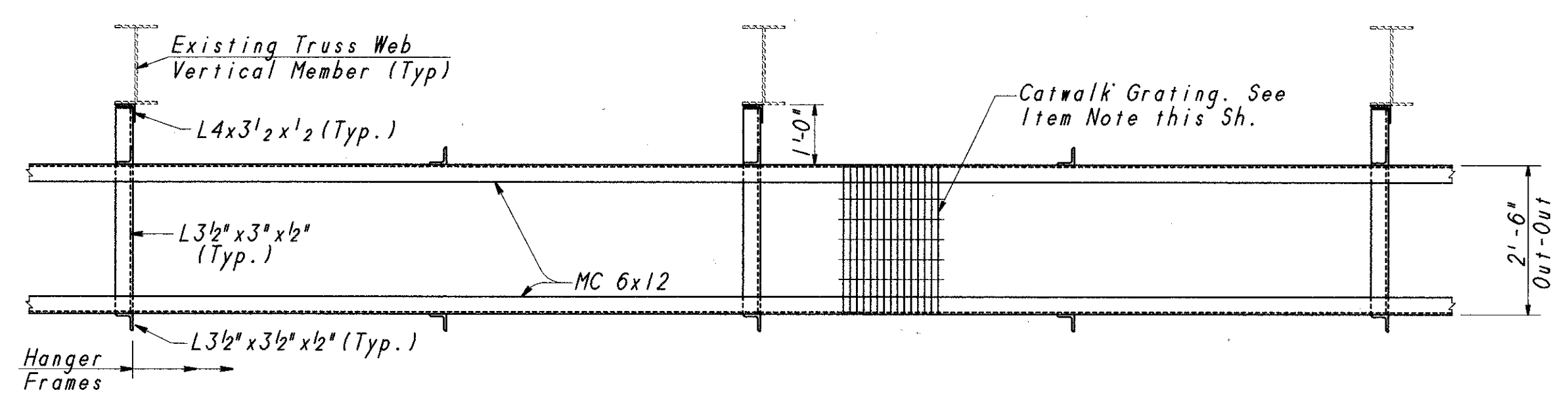
NOTE:
 Work this sheet with CATWALK PLAN AND ELEVATION 1/2, Sh. 100/108

Pflum, Klausmeier & Gehrmann Consultants		101/108
CATWALK PLAN AND ELEVATION 2/2		
BRIDGE No. HAM-471-0025 (Columbia Parkway Viaduct Over I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN ED	TRACED WDD
CHECKED WDD	REVIEWED IEH April 96	DATE REVISION

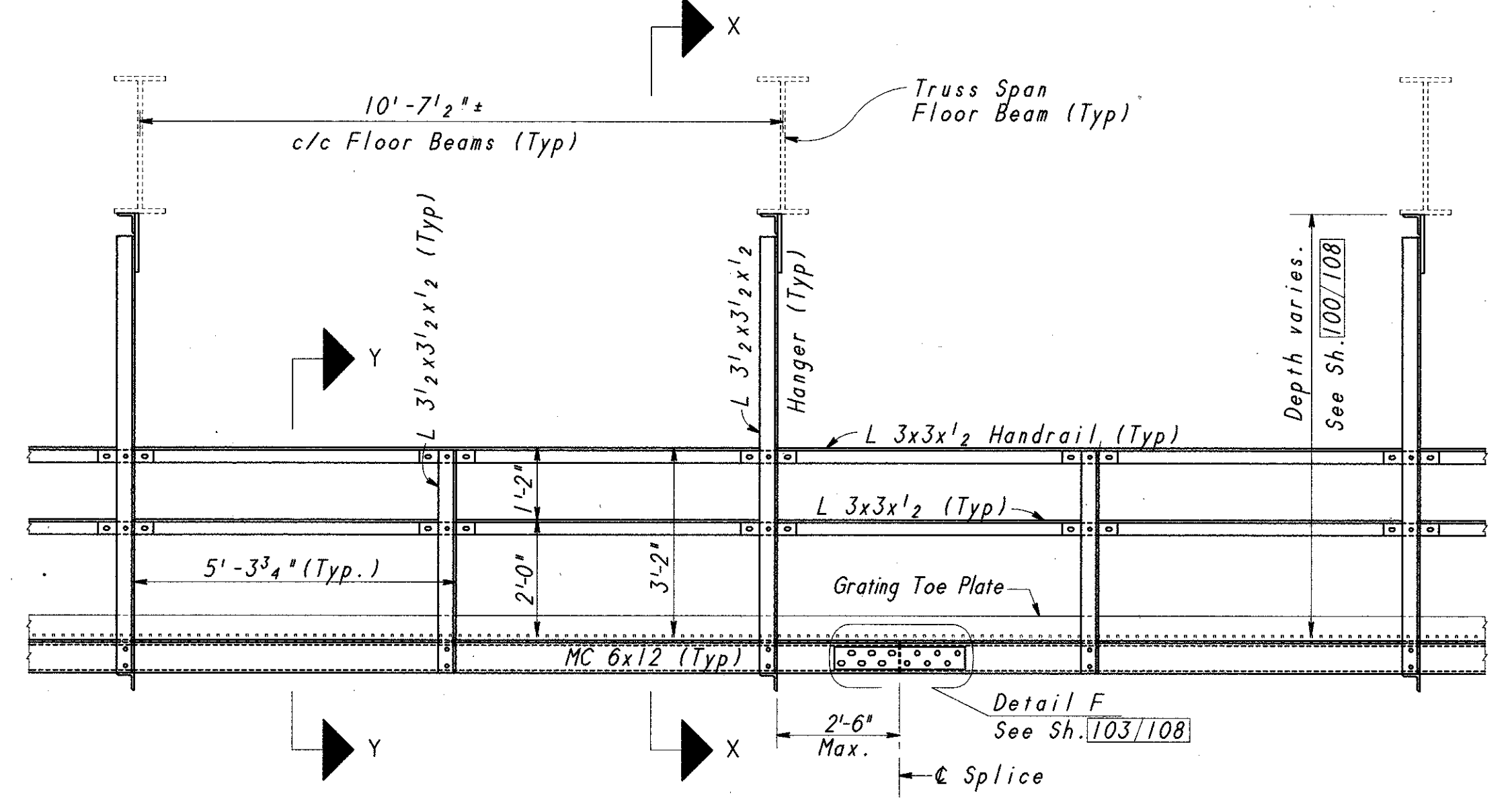
WALKY2 Scale 1/6

HAM-471-00.25

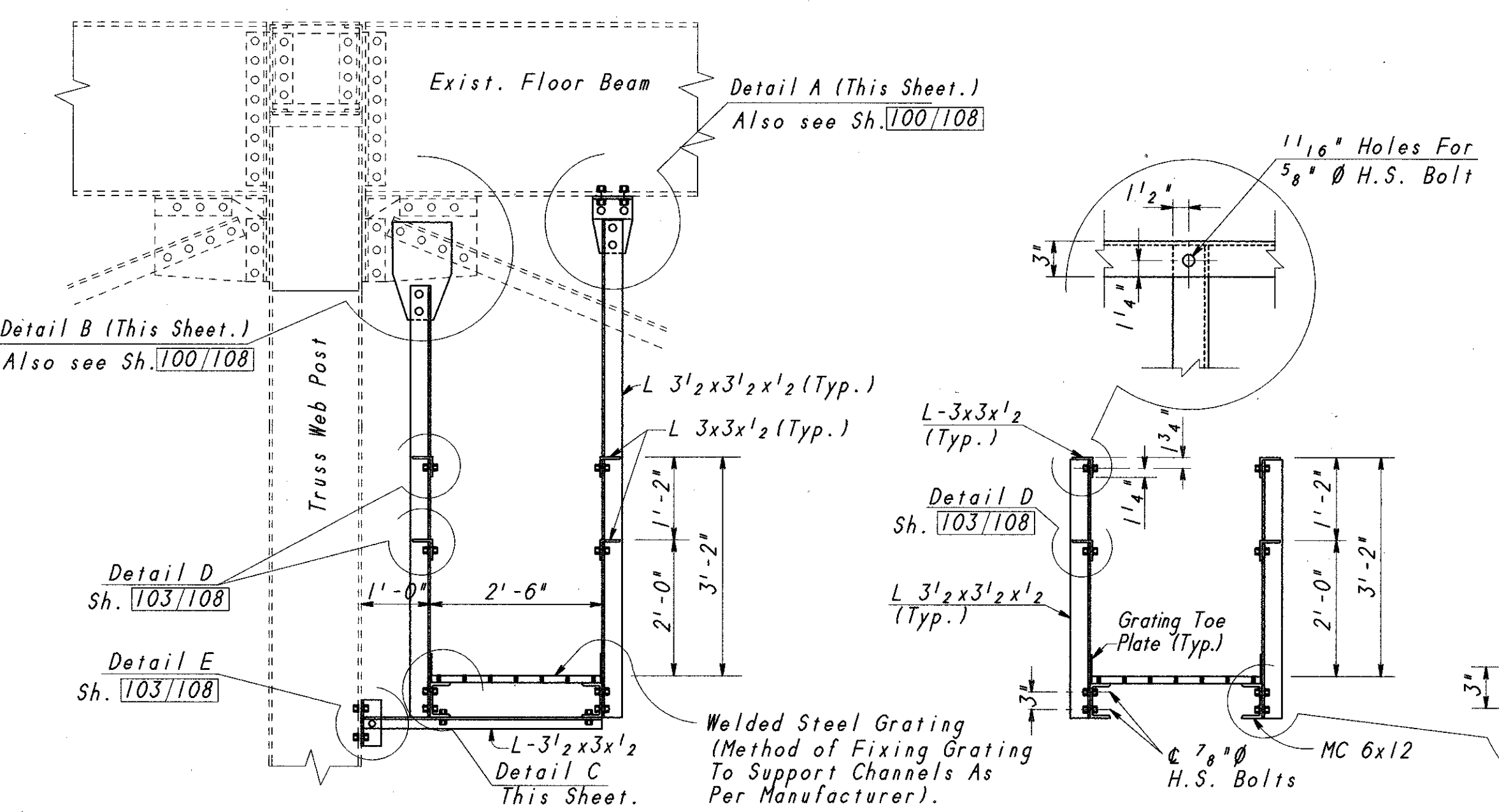
LEGEND
 Existing Structure
 New Work



CATWALK PARTIAL PLAN
(FOR THE FULL PLAN)

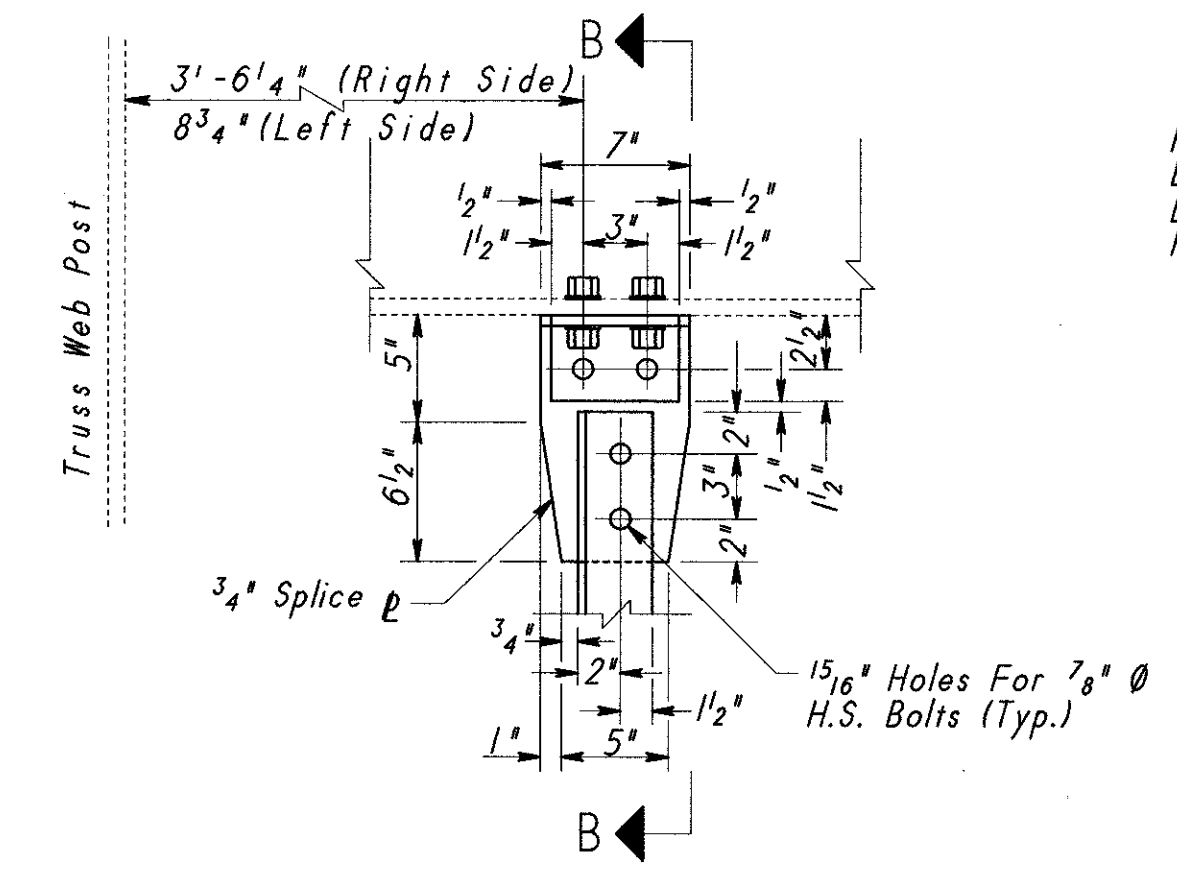


CATWALK PARTIAL ELEVATION
(FOR THE FULL ELEVATION)

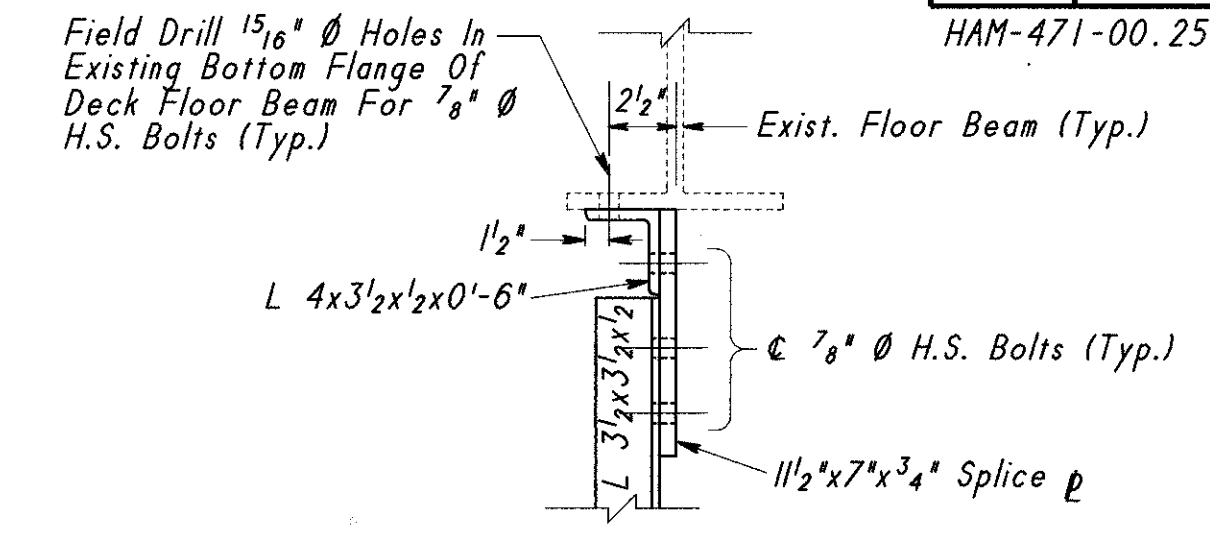


SECTION X-X

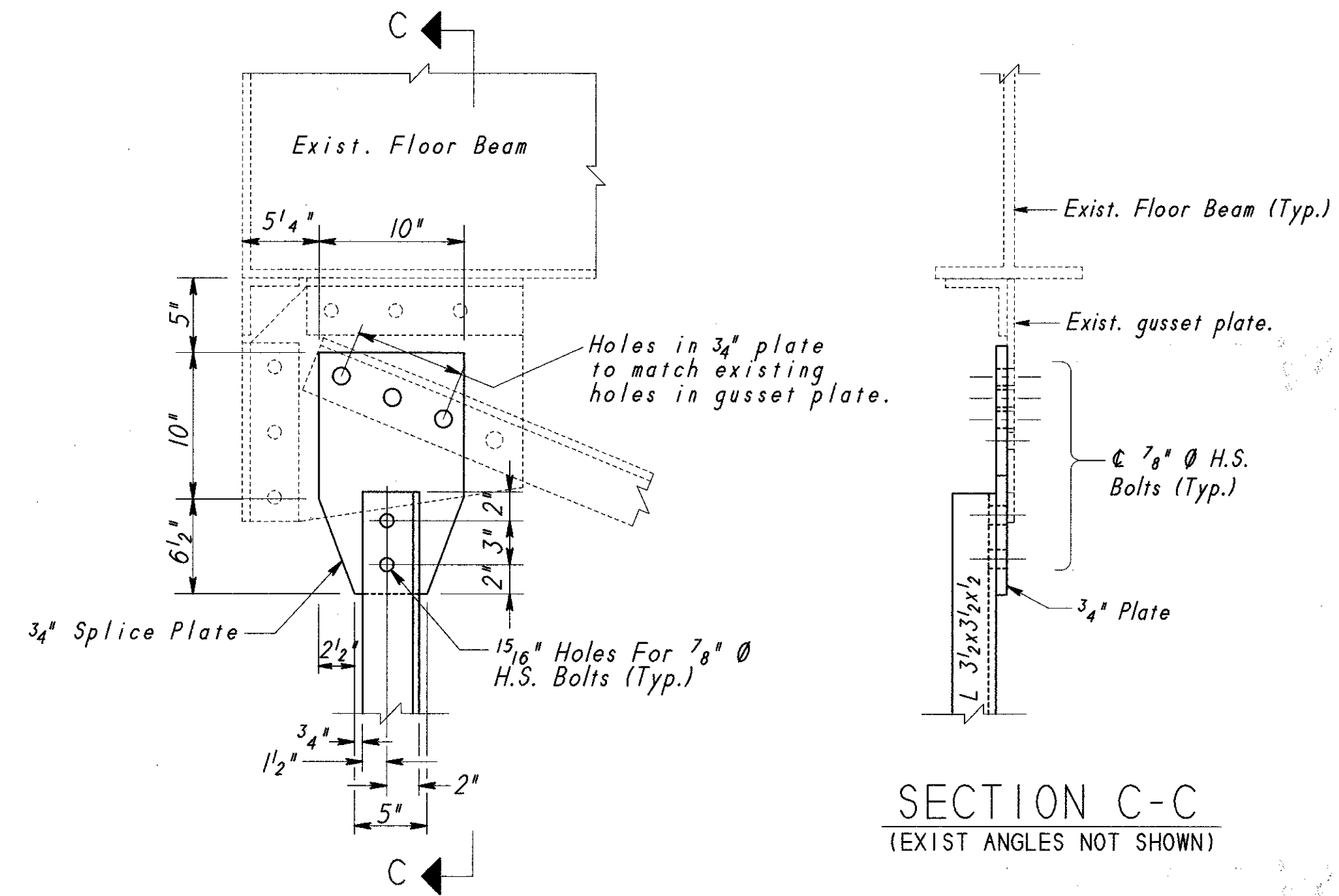
SECTION Y-Y



DETAIL A
(Right Side Shown, Left Side Similar)

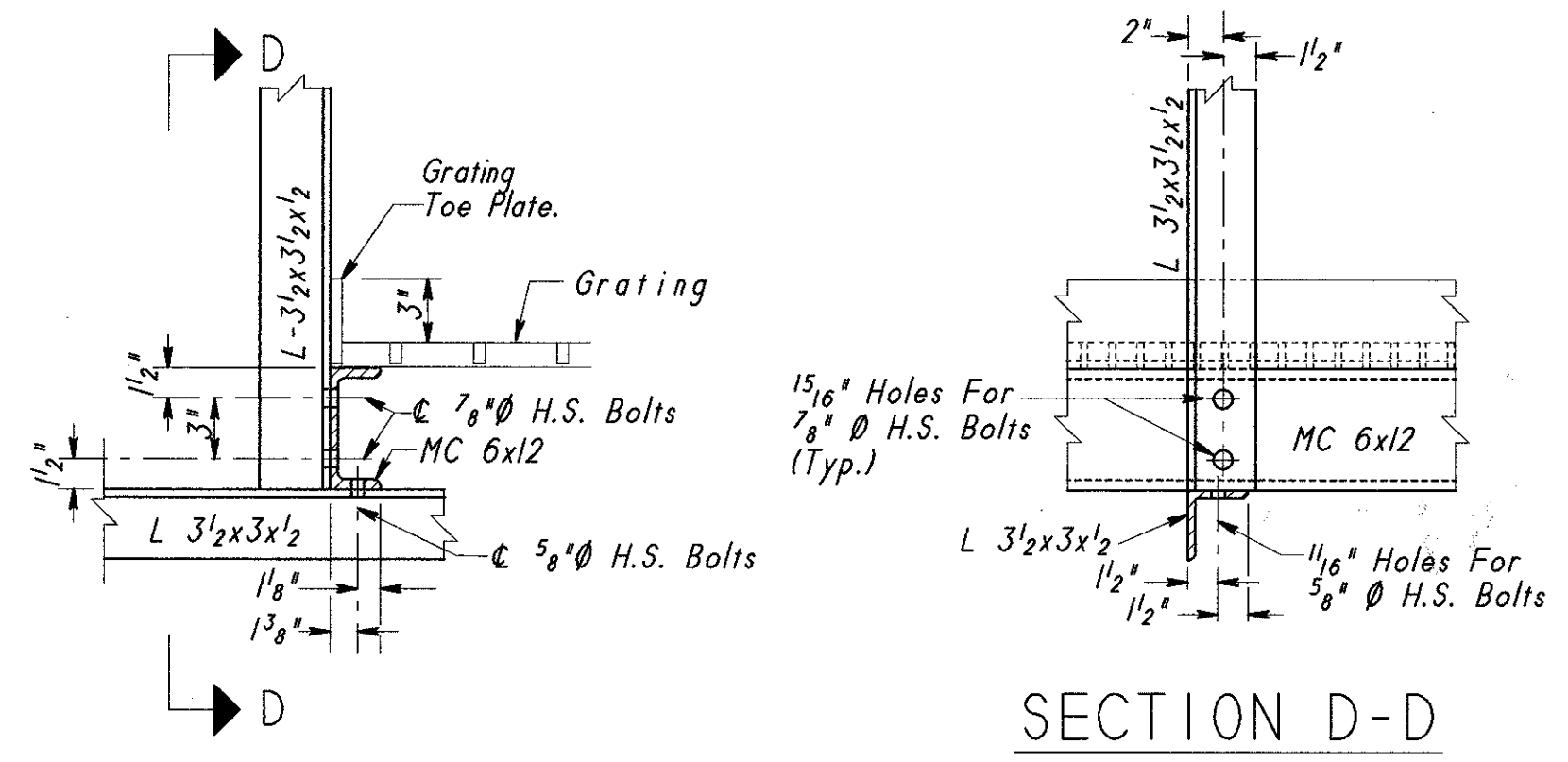


SECTION B-B



DETAIL B

SECTION C-C
(EXIST ANGLES NOT SHOWN)



DETAIL C
(OTHER SIDE SIMILAR)

SECTION D-D

ITEM 513. - CATWALK GRATING.
 This item includes all labor, material, equipment, tools, and incidentals necessary to fabricate and install a welded steel grating system in the areas and on the length and position described on Structural Steel Catwalk Plan Sheets [100/108] thru [108/108].

Prior to grating installation, the Contractor shall inspect supports for correct size, layout and alignment, and take field measurements prior to preparation of the required shop drawings. Prior to grating installation the Contractor shall submit detailed shop drawings to the Engineer for review and approval. After installation, the Contractor shall submit as-built drawings to the Engineer to ensure that the drawings depict the steel grating as actually incorporated into the work. The steel grating described in this item shall be capable of a load bearing capacity of 85 pounds per square foot on a span indicated on the catwalk plan, and shall not exceed a depth of 1 inch, and a maximum deflection of 0.112 inches on a 2'-6" span. The shop drawings prepared by the Contractor shall include plans, elevations, and details of sections and connections, as well as type and location of all fasteners. The Contractor shall submit the grating manufacturer's specifications, load tables, anchor details and standard installation details. The grating installation shall be in accordance with the approved shop drawings and the standard installation clearances as recommended by the National Association of Architectural Metal Manufacturers (NAAMM) Metal Bar Grating Manual. The Contractor shall perform all cutting and fitting required for installation. The item includes the use of anchorage devices (saddle clips, grating clamps, anchor blocks, plank clips, Z clips, countersunk land) toe plates and fasteners to secure grating to the supporting members. Grating shall be placed such that cross bars align.

TYPE AND MATERIAL: The grating shall be type 19W4 light duty welded steel grating 1"x1/8" bar, 5.15psf as manufactured by Ohio Gratings Inc., 5299 Southway, S.W., Canton, Ohio 44706 or type GW-100A 1"x1/8" bar, 5.2psf as manufactured by McNichols Co., 489 NEO Parkway, Cleveland, Ohio 44128-3195, and shall comply with the applicable provisions of NAAMM Metal Bar Grating Manual and the American National Standard Institute (ANSI) MGB 531 Light Duty Steel Grating. The material shall be Light Duty Steel ASTM A569 hot rolled carbon steel sheet and strip.

FINISH: Galvanized

METHOD OF PAYMENT: The pay quantity for grating will be based on the actual area out-to-out of grating installed, inspected, approved, and accepted by the Engineer.

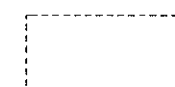

BASIS OF PAYMENT: Payment will be made at the Contract unit price bid per square foot for Item 513, CATWALK GRATING.

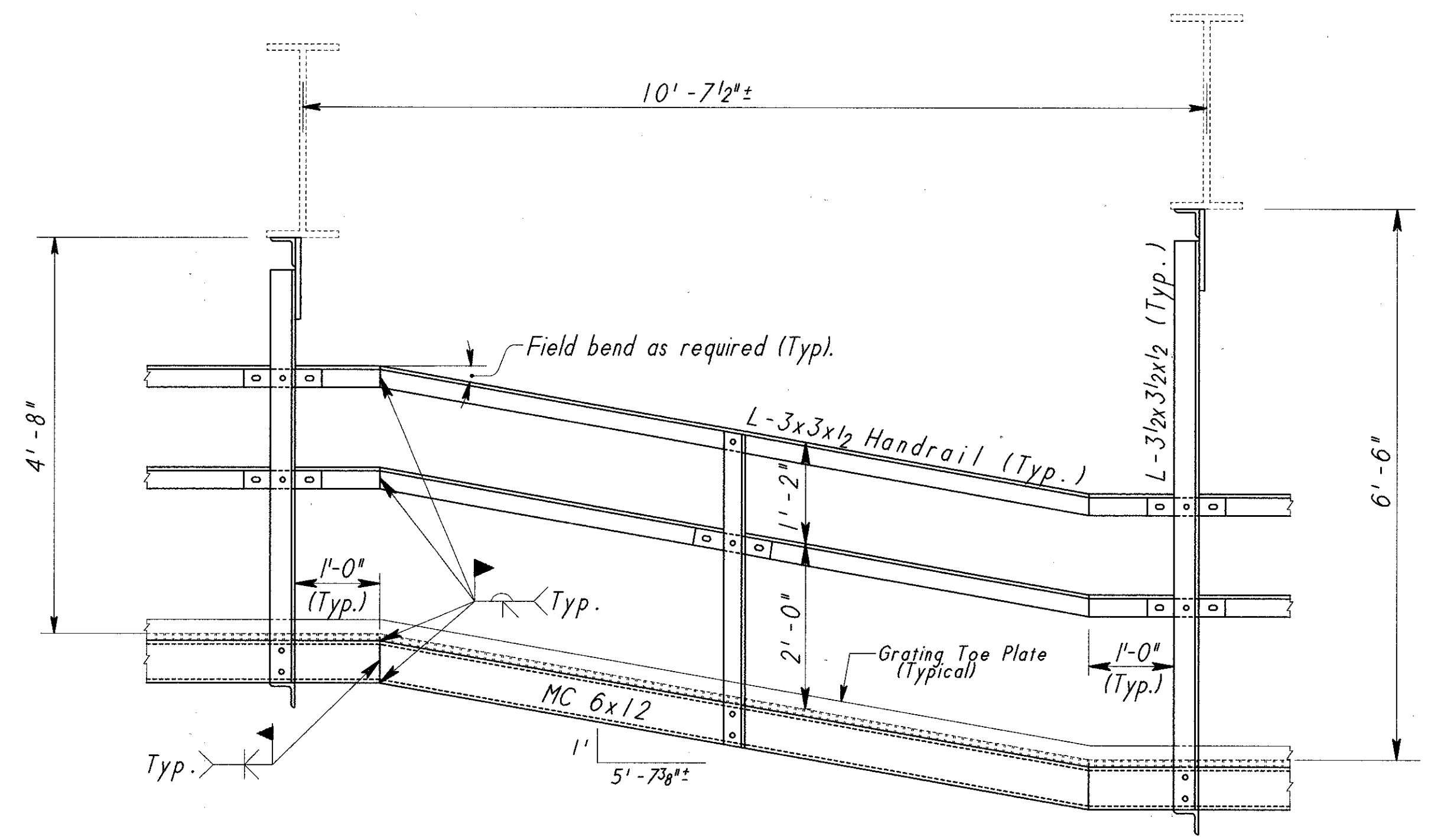
NOTES:
 1. - See General Notes, Sh. 7/108 Thru Sh. 11/108.

Pflum, Klausmeier & Gehrmann		102/108
CONSULTANTS		
CATWALK DETAILS 1/2		
BRIDGE No. HAM-471-0025 (Columbia Parkway Viaduct Over I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	ED	REVIEWED
DRAWN	PLF	DATE
TRACED	WDD	1EH April 96
CHECKED		REVISIONS

WALKYSEC Scale 1:333

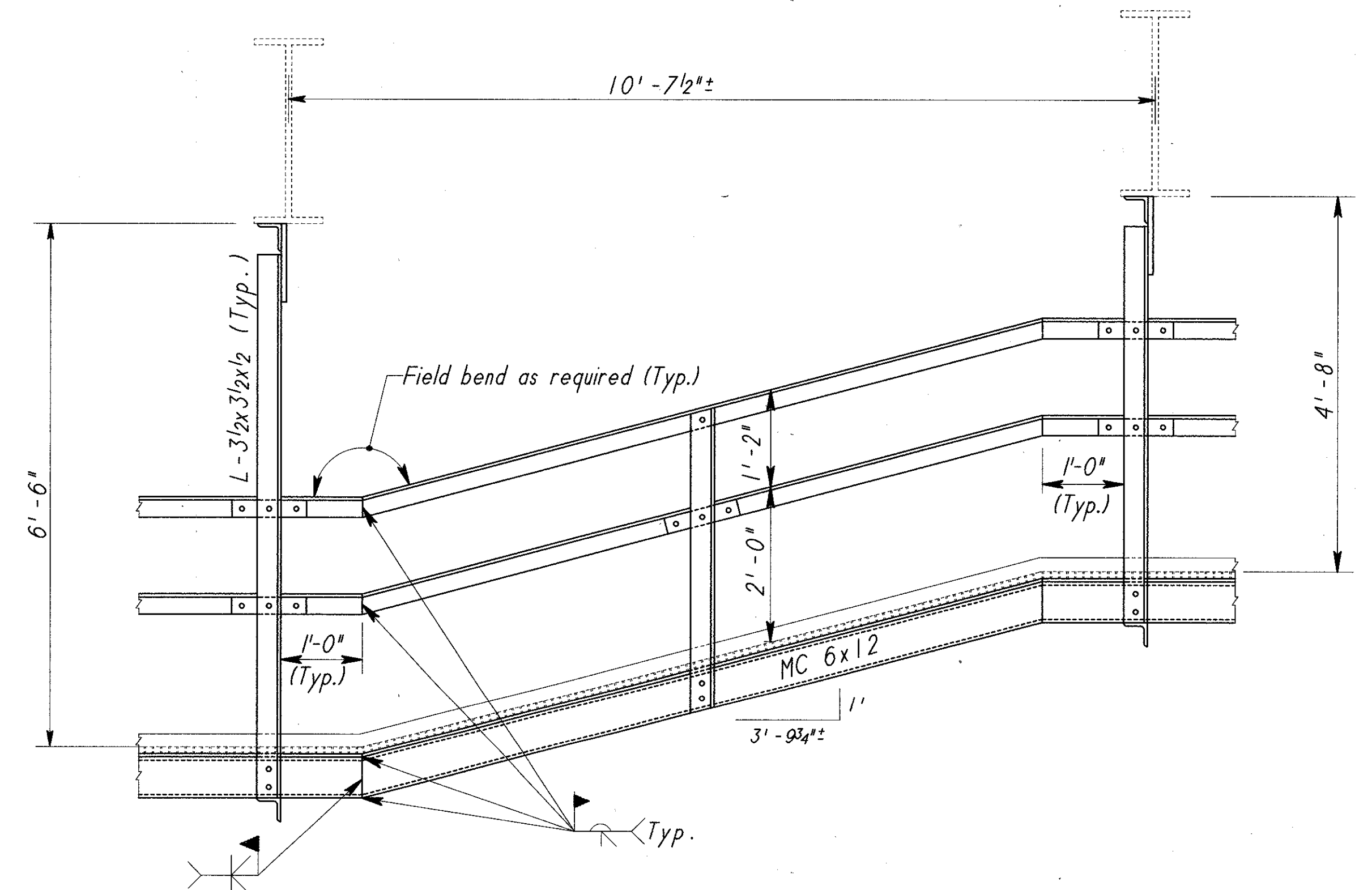
LEGEND

-  Existing Structure
-  New Work



DETAIL FOR BEND AREA NEAR PIER 6

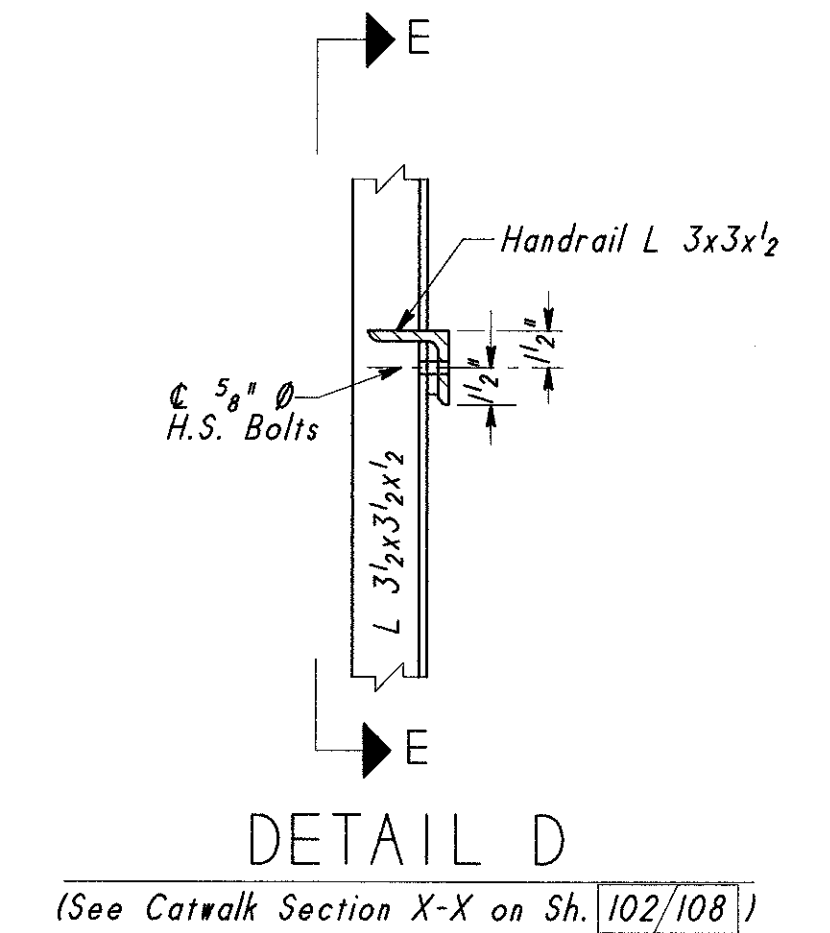
The Contractor shall field verify the required bend configuration, and incorporate the proper detail with the catwalk required shop drawings for approval by the Engineer (Typical).



DETAIL FOR BEND AREA NEAR PIER 11

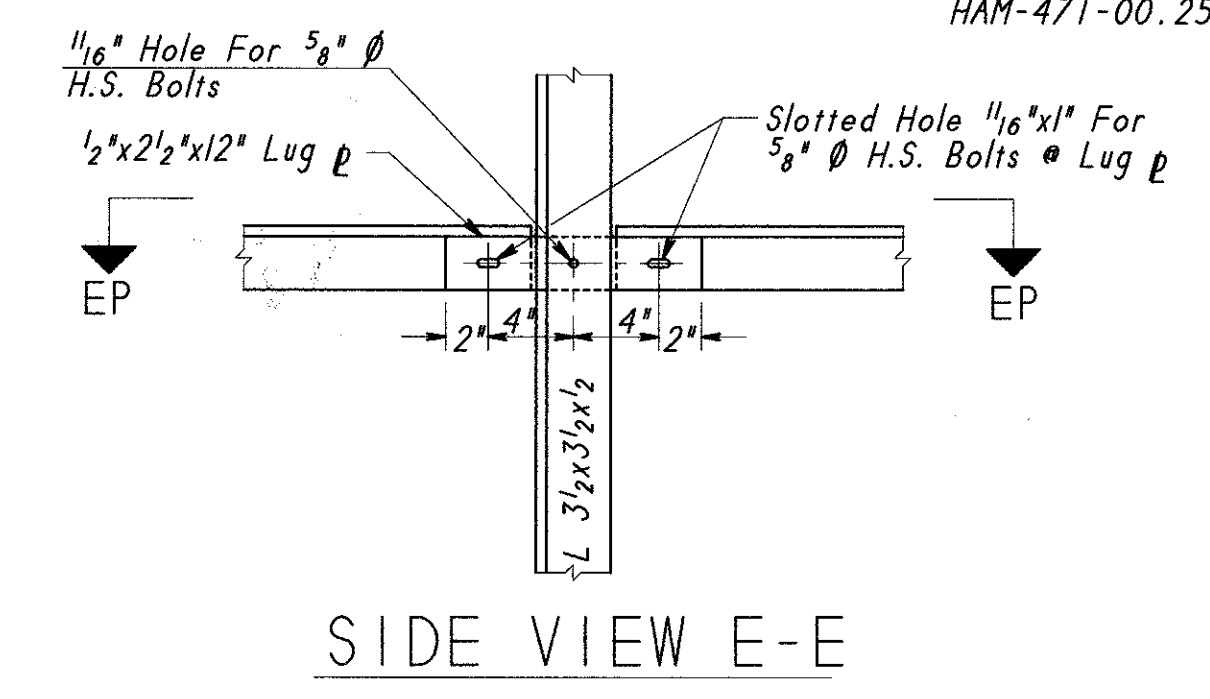
NOTES:

- 1.-Work this sheet with Sh. 102/108
- 2.-See GENERAL NOTES, Sh. 7/108 Thru Sh. 11/108

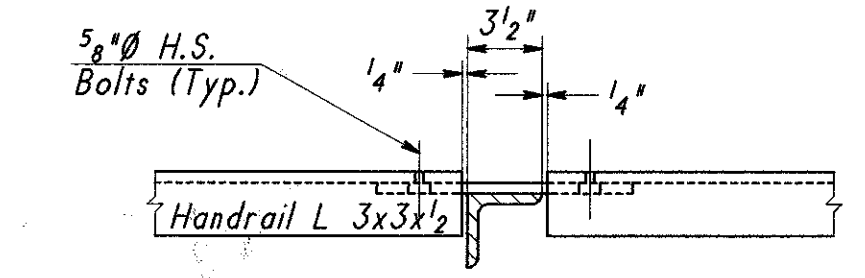


DETAIL D

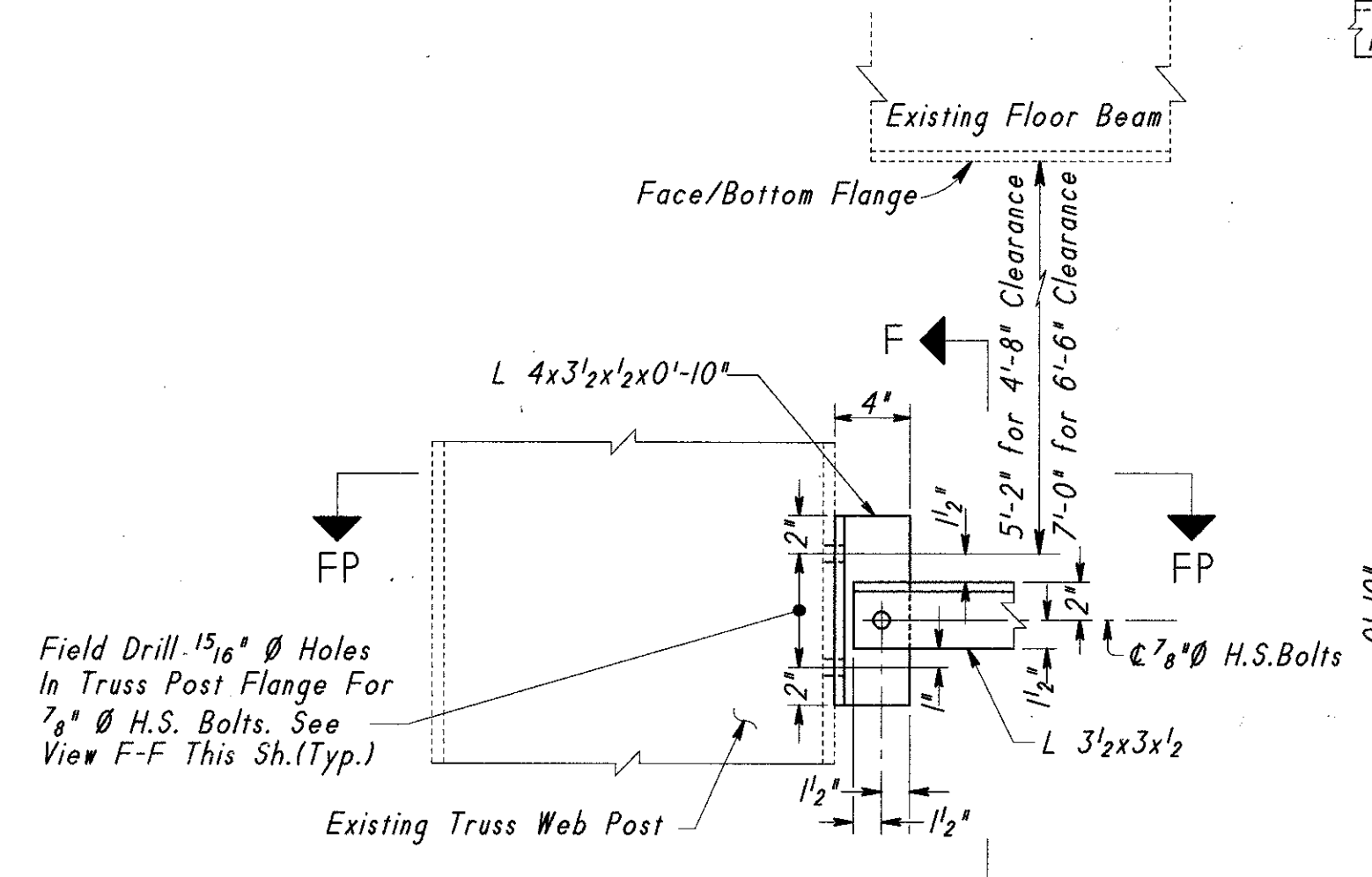
(See Catwalk Section X-X on Sh. 102/108)



SIDE VIEW E-E

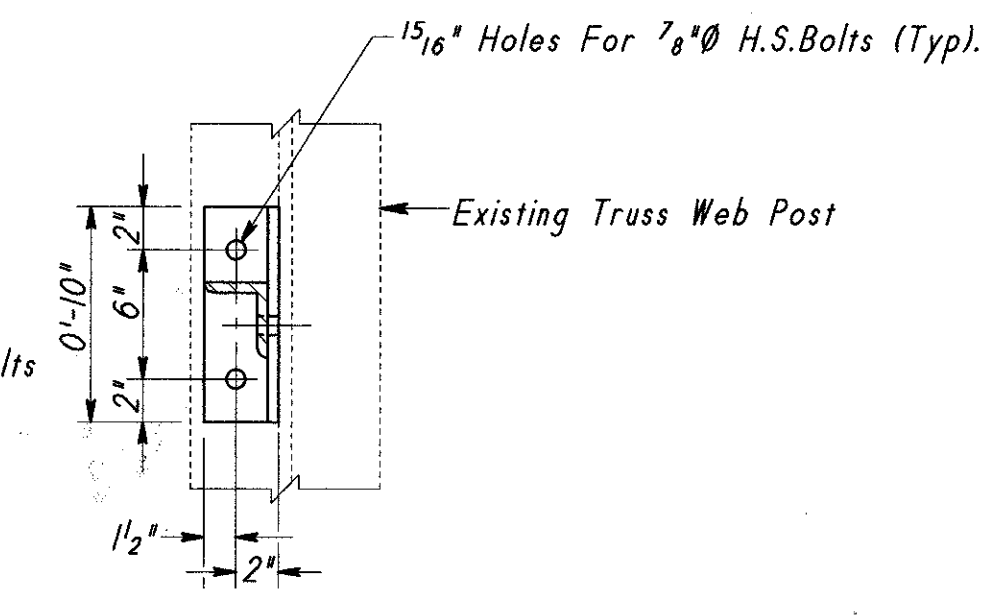


PLAN VIEW EP-EP

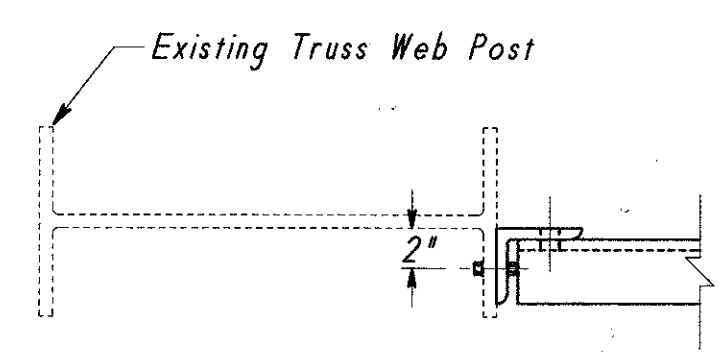


DETAIL E

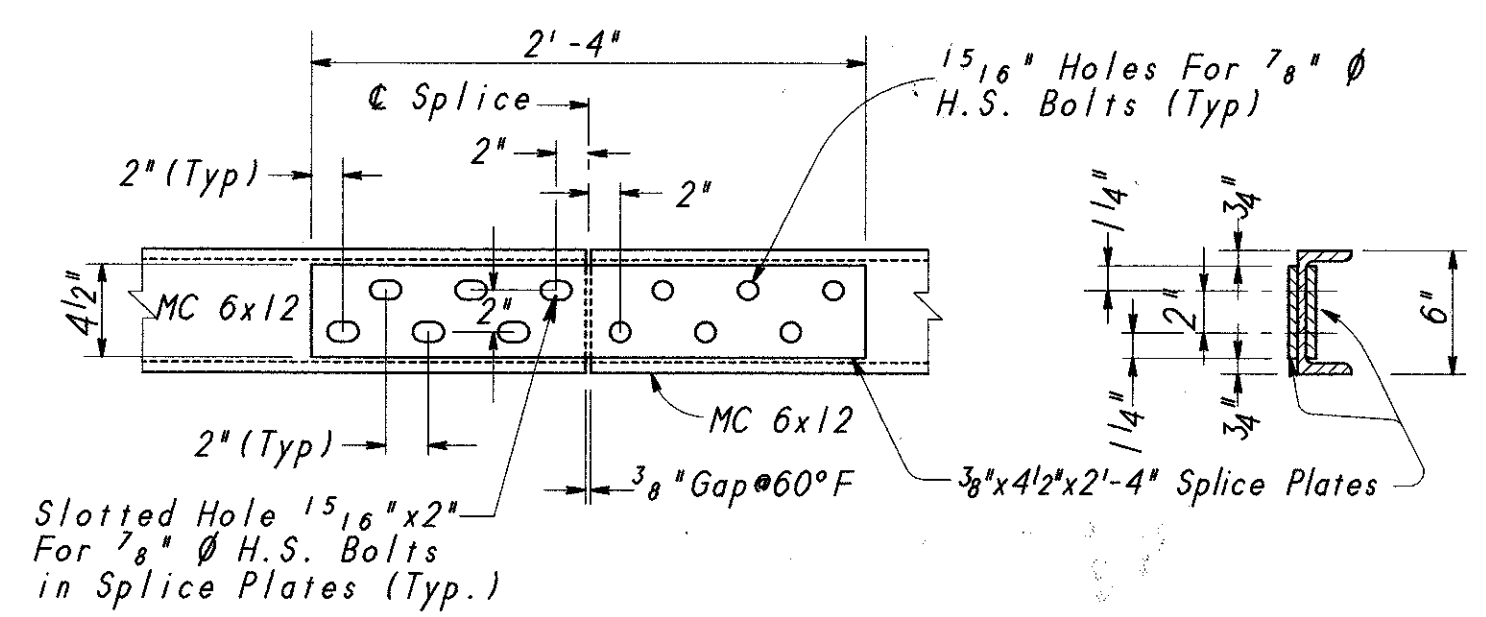
(See Catwalk Section X-X on Sh. 102/108)



VIEW F-F



VIEW FP-FP



DETAIL F

OPTIONAL SPLICE DETAIL*

(See Catwalk Partial Elevation on Sh. 102/108)

* The splicing method shown is optional. The Contractor may consider alternate methods of splicing the catwalk main channel, and submit details which shall be included with the catwalk shop drawing, to the Engineer, for approval.

Plum, Klausmeier & Gehrum CONSULTANTS		103/108
CATWALK DETAILS 2/2		
BRIDGE No. HAM-471-0025 (Columbia Parkway Viaduct Over I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96



HAMILTON COUNTY ENGINEERING

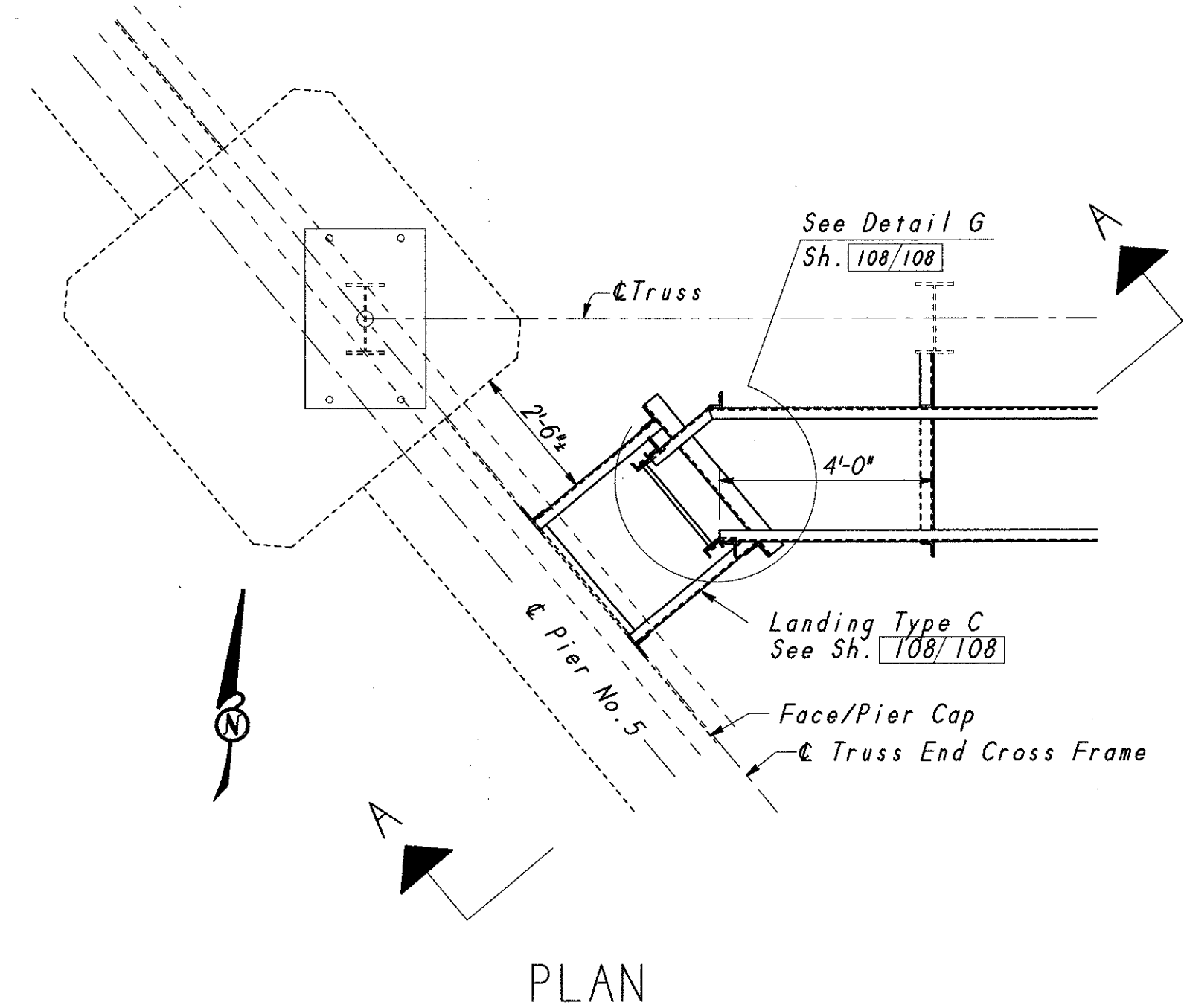
F.H.W.A. REGION	STATE	PROJECT	
5	OHIO		

178
182

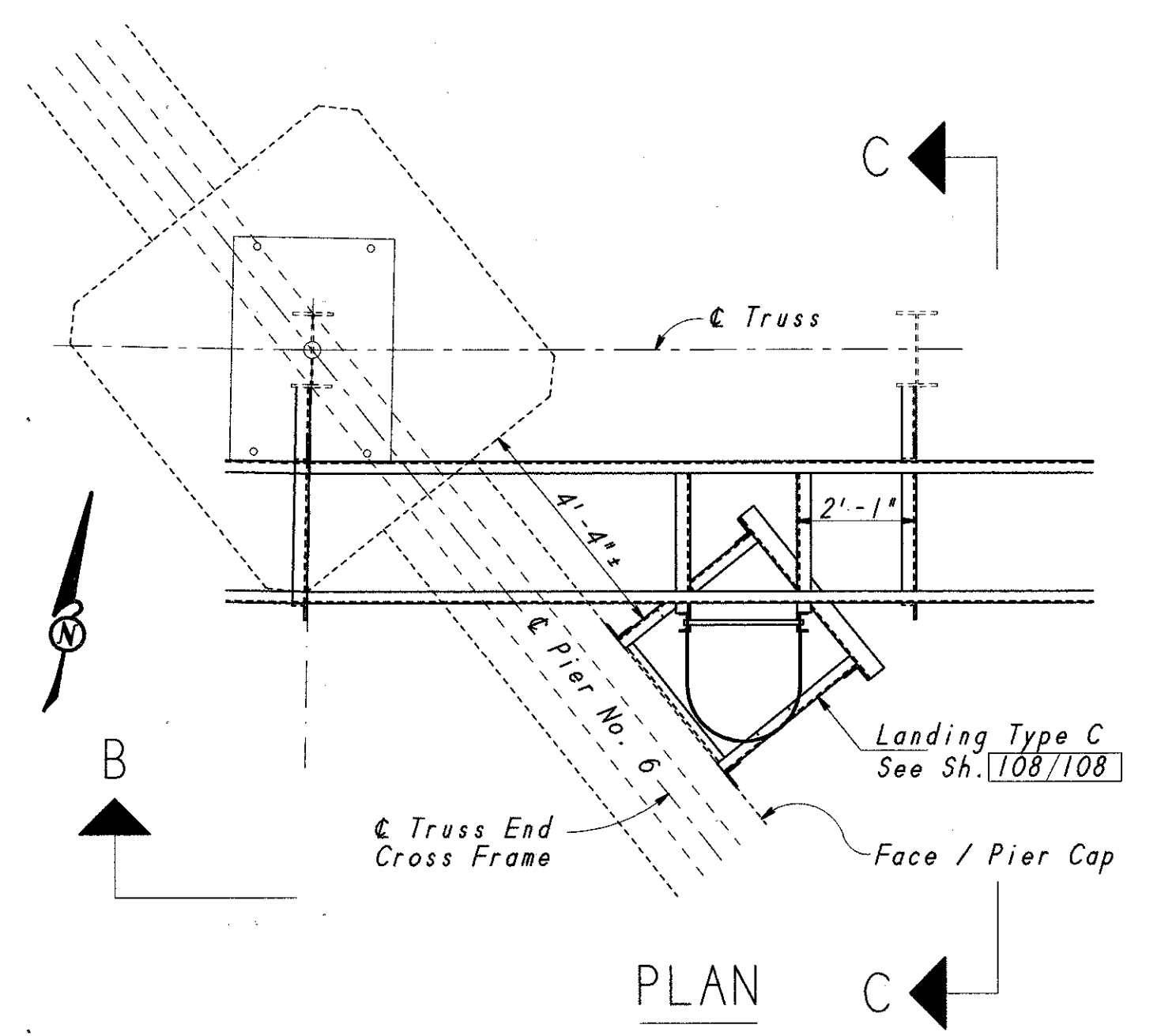
HAM-471-00.25

LEGEND

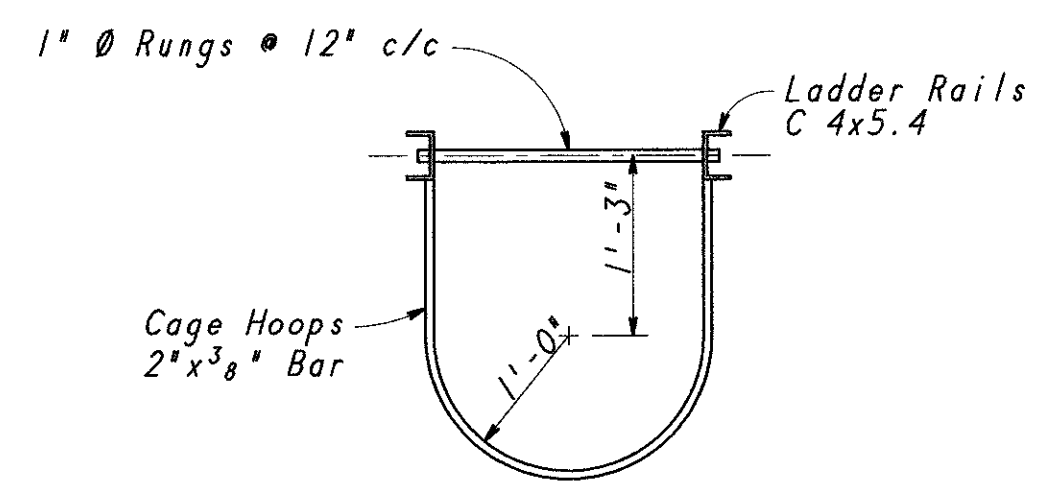
-  Existing Structure
-  New Work



PLAN

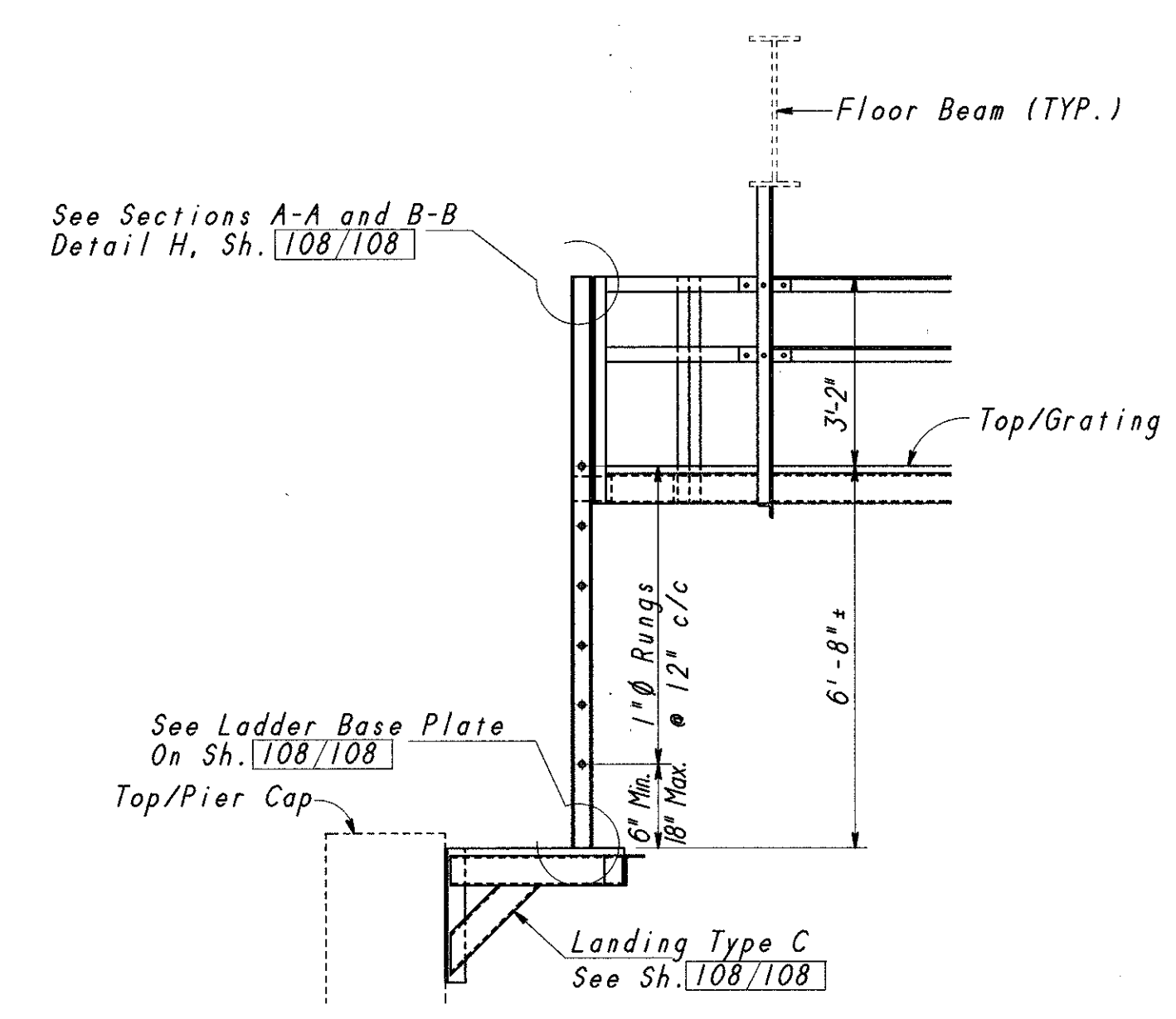


PLAN



LADDER TYPICAL SECTION

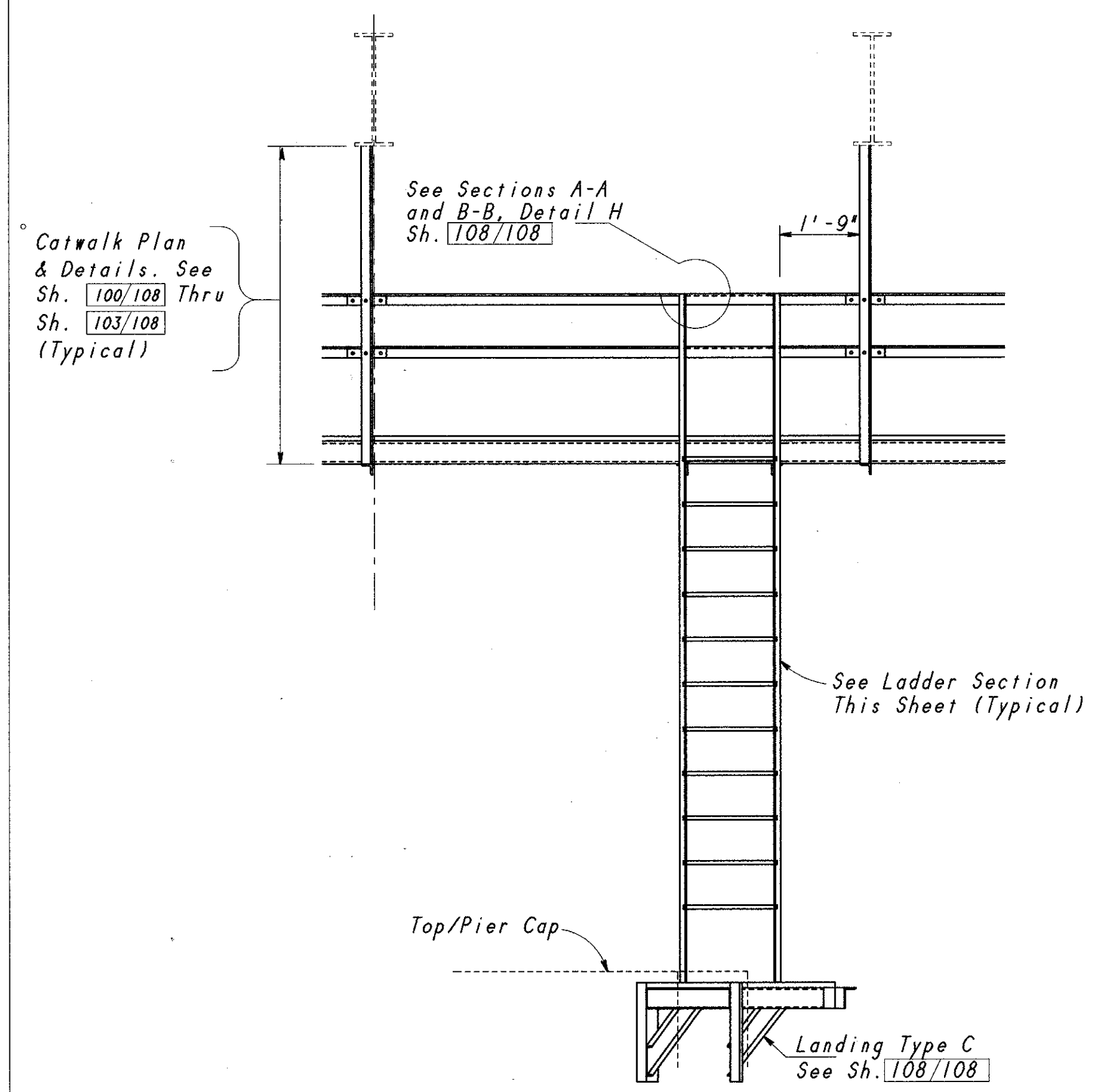
- NOTES:
- 1.- Work this sheet with Sheets [100/108], [101/108] and [108/108].
 - 2.- Landing Platforms Safety Rails not shown for simplicity. See Sh. [108/108].



VIEW A-A

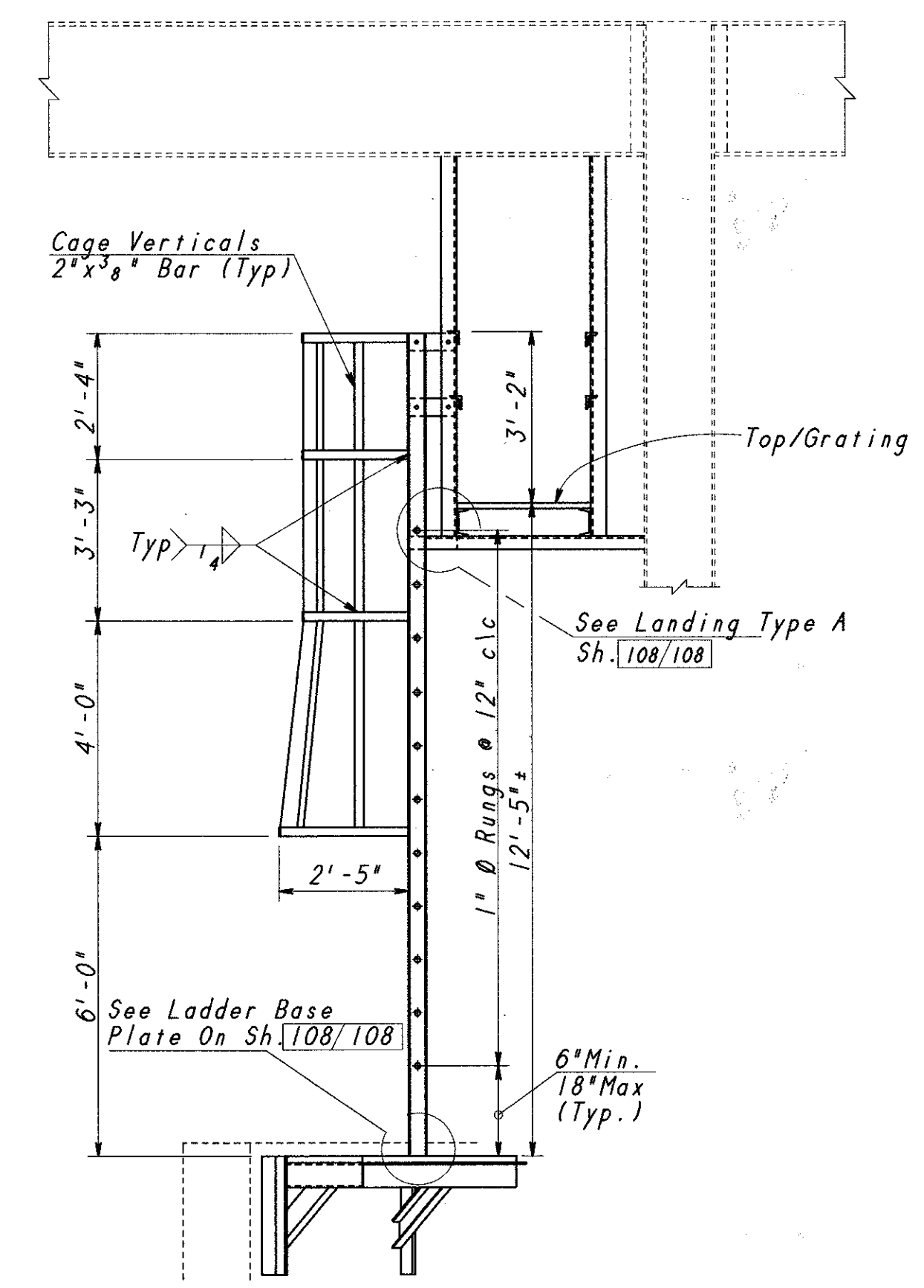
SERVICE LADDER NO. 1

NOTE: Service Ladder No.1 Does Not Require Safety Hoops.




VIEW B-B

SERVICE LADDER NO. 2



VIEW C-C

 Pflum, Klausmeyer & Gehrmann Consultants CINCINNATI OHIO		104/108
CATWALK SERVICE LADDER No. 1 (PIER NO. 5) No. 2 (PIER NO. 6) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96

LS142 Scale 2:606

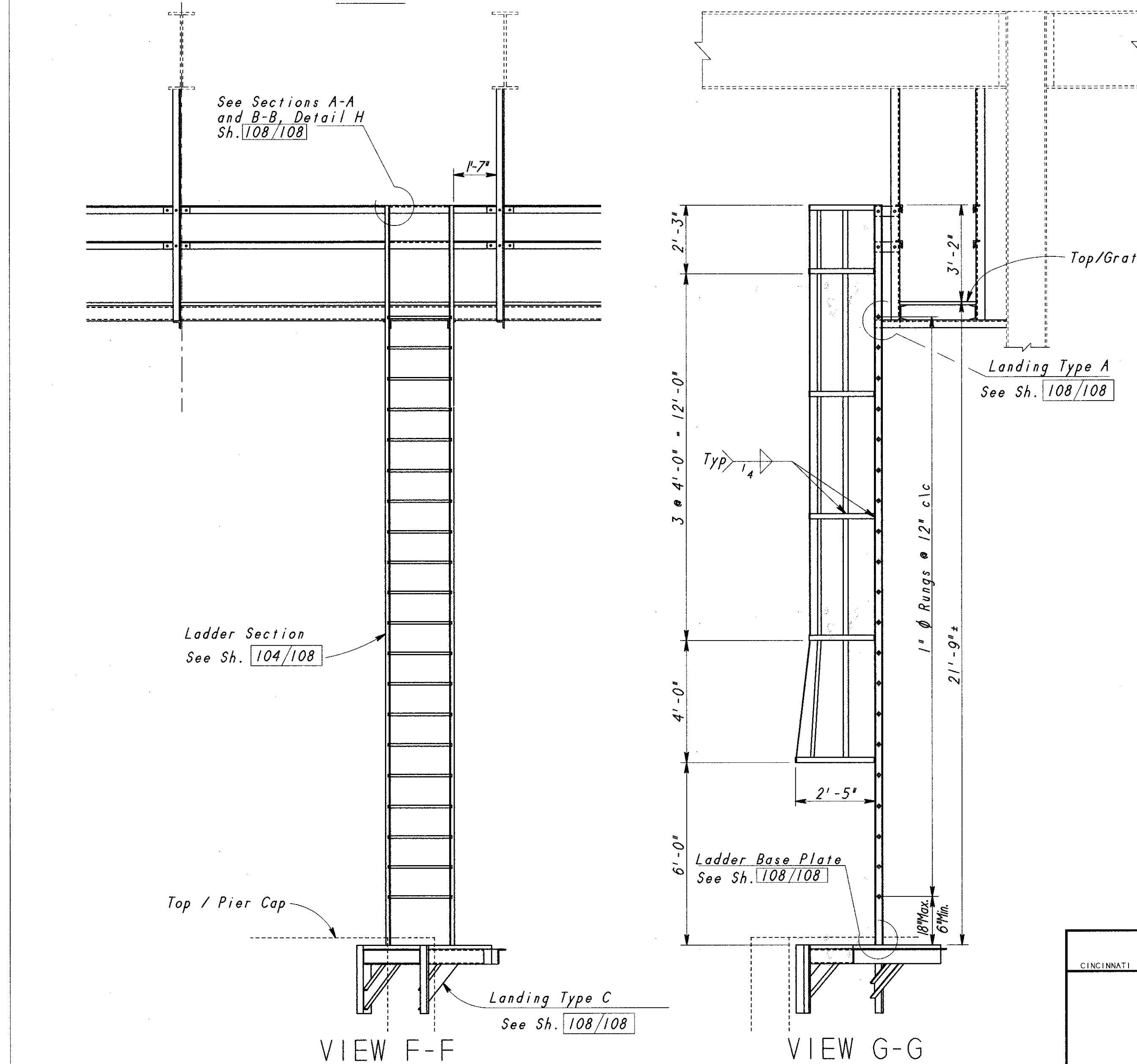
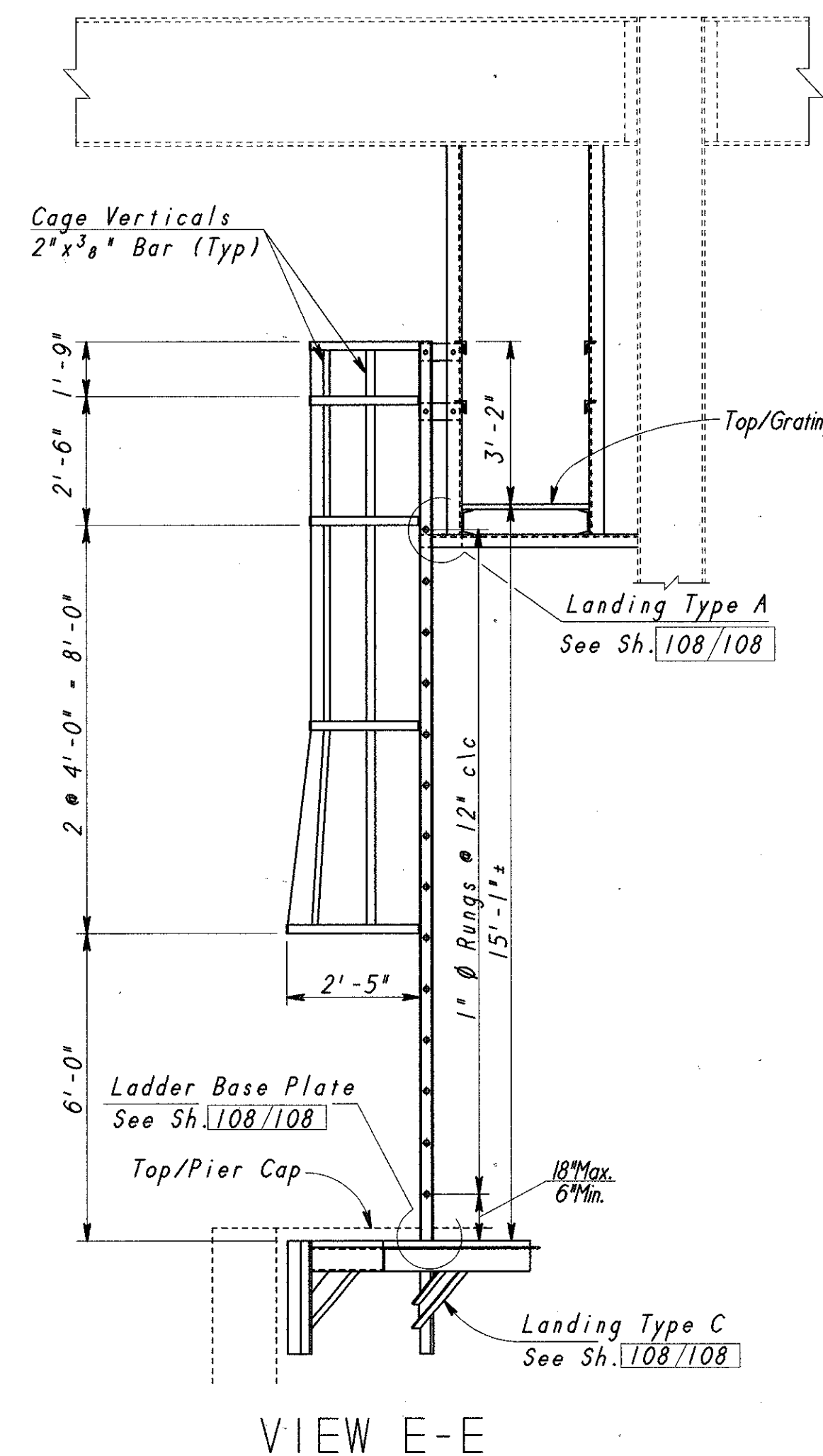
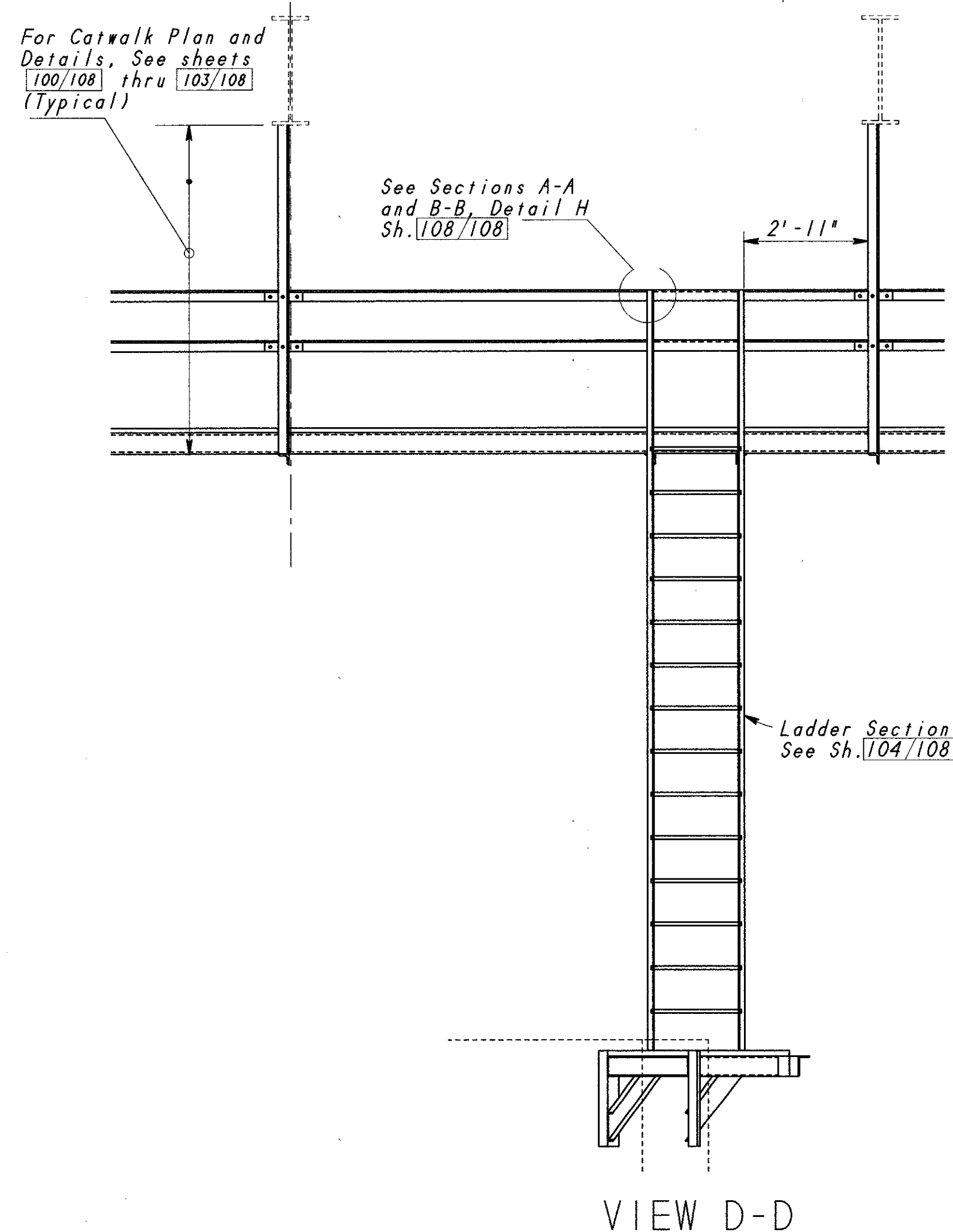
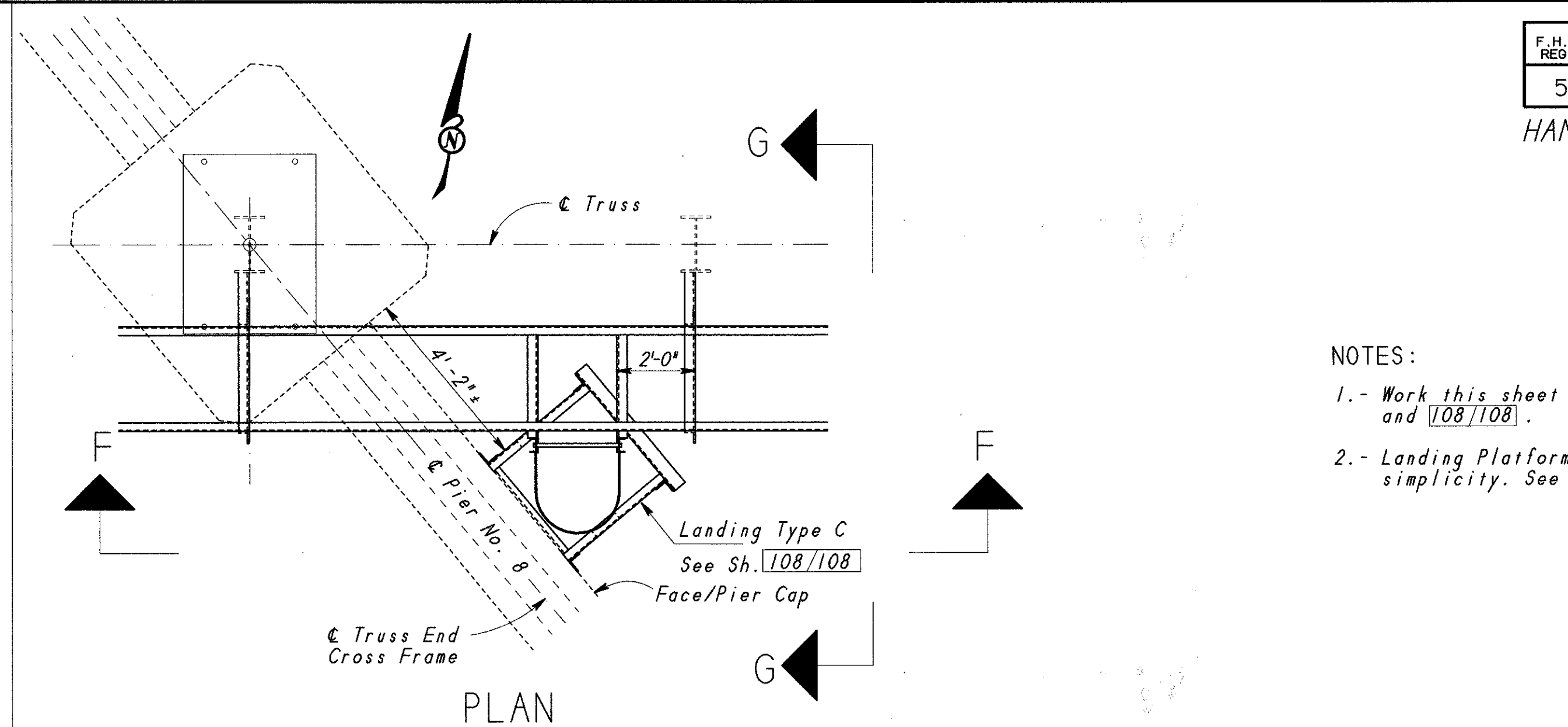
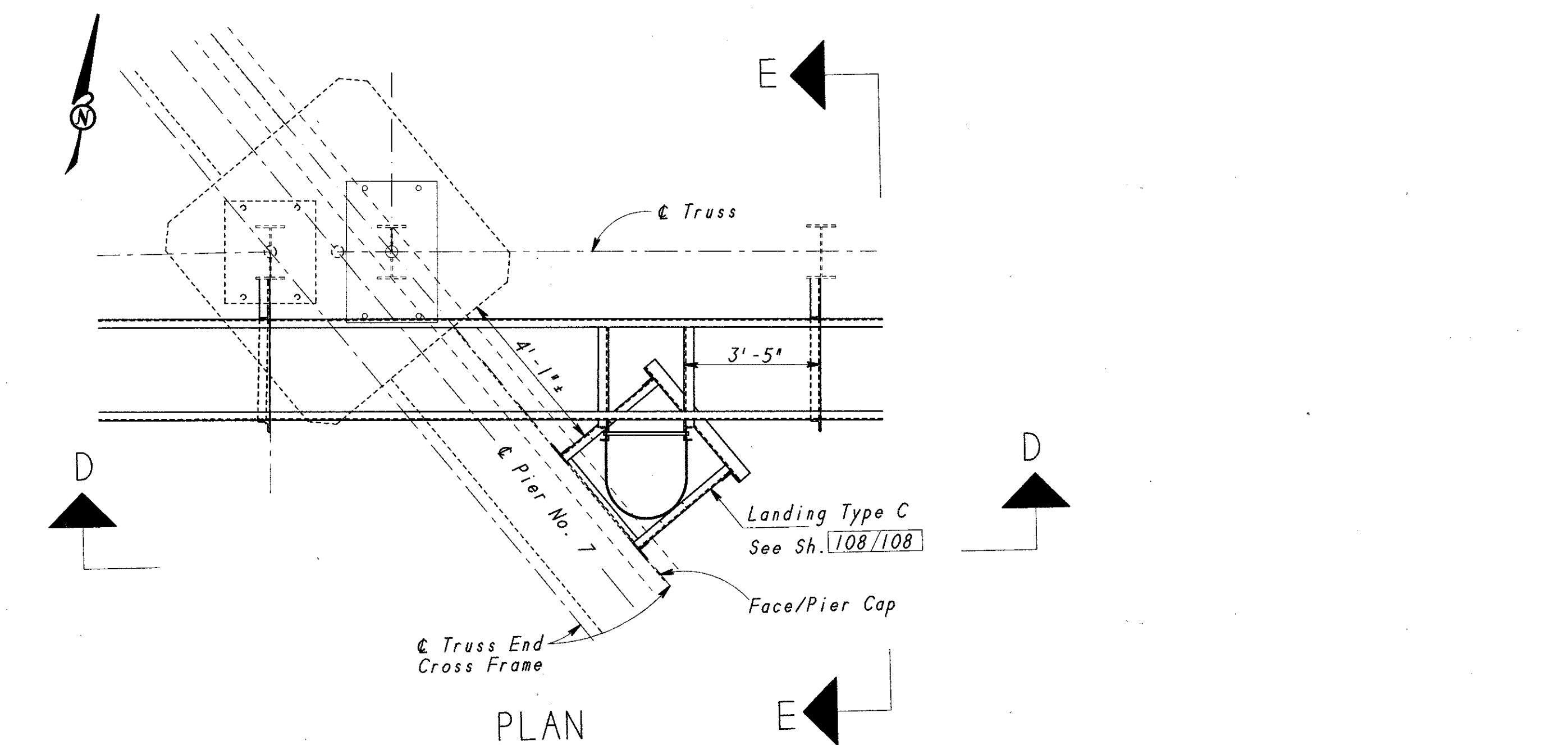
F.H.V.A. REGION	STATE	PROJECT	
5	OHIO		

179
182

HAM-471-00.25

NOTES:

- 1.- Work this sheet with Sheets [100/108], [101/108], and [108/108].
- 2.- Landing Platform Safety Rails not shown for simplicity. See Sh. [108/108] (Typ.)



LEGEND

	Existing Structure
	New Work

SERVICE LADDER NO. 3

SERVICE LADDER NO. 4

Pflum, Klausmeyer & Gehrmann Consultants		105/108
CATWALK SERVICE LADDER		
No. 3 (PIER NO. 7)		
No. 4 (PIER NO. 8)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
		Sta. 47+16.19
DESIGNED	DRAWN	TRACED
ED	PLF	WDD
CHECKED	REVIEWED	DATE
IEH	April 96	

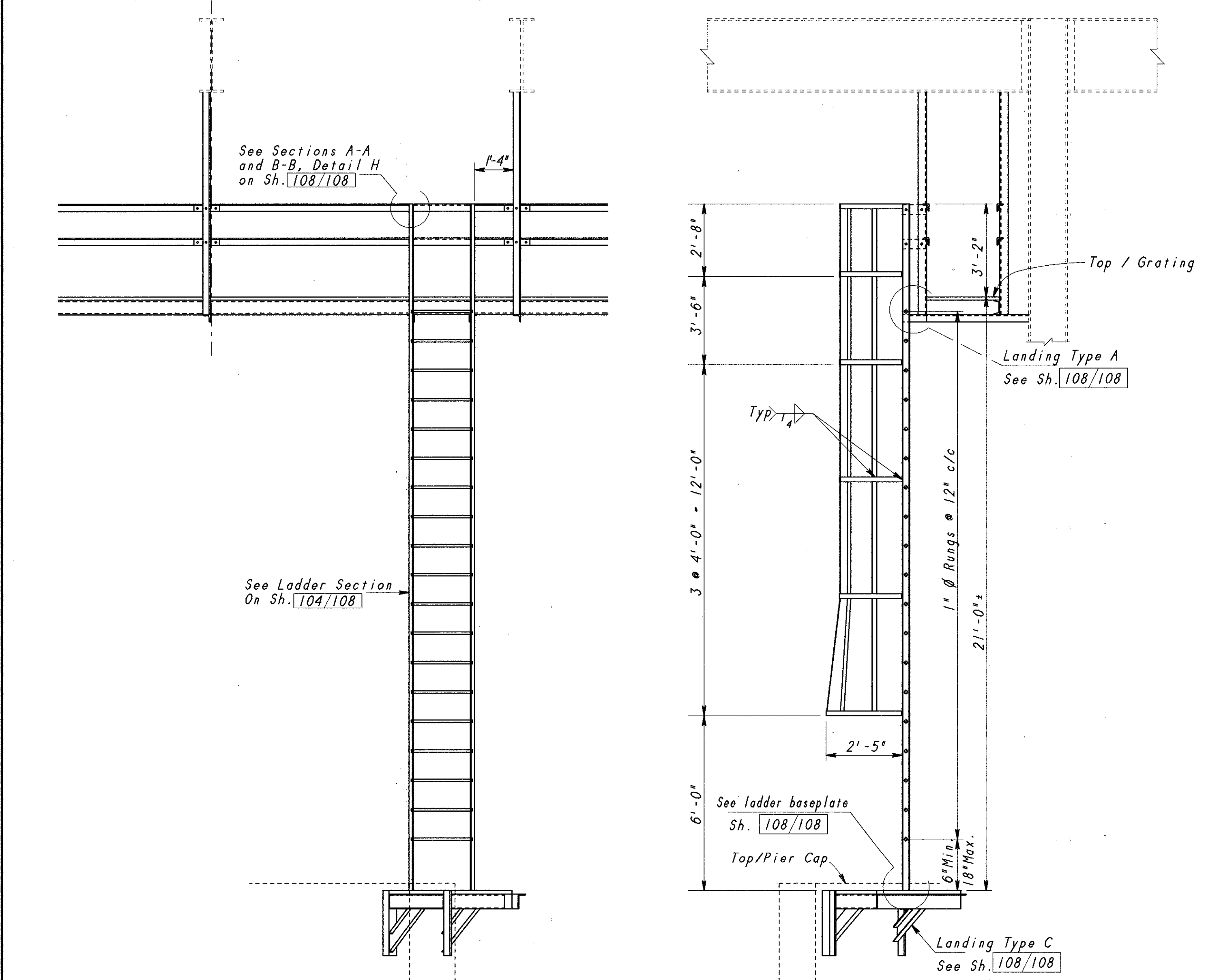
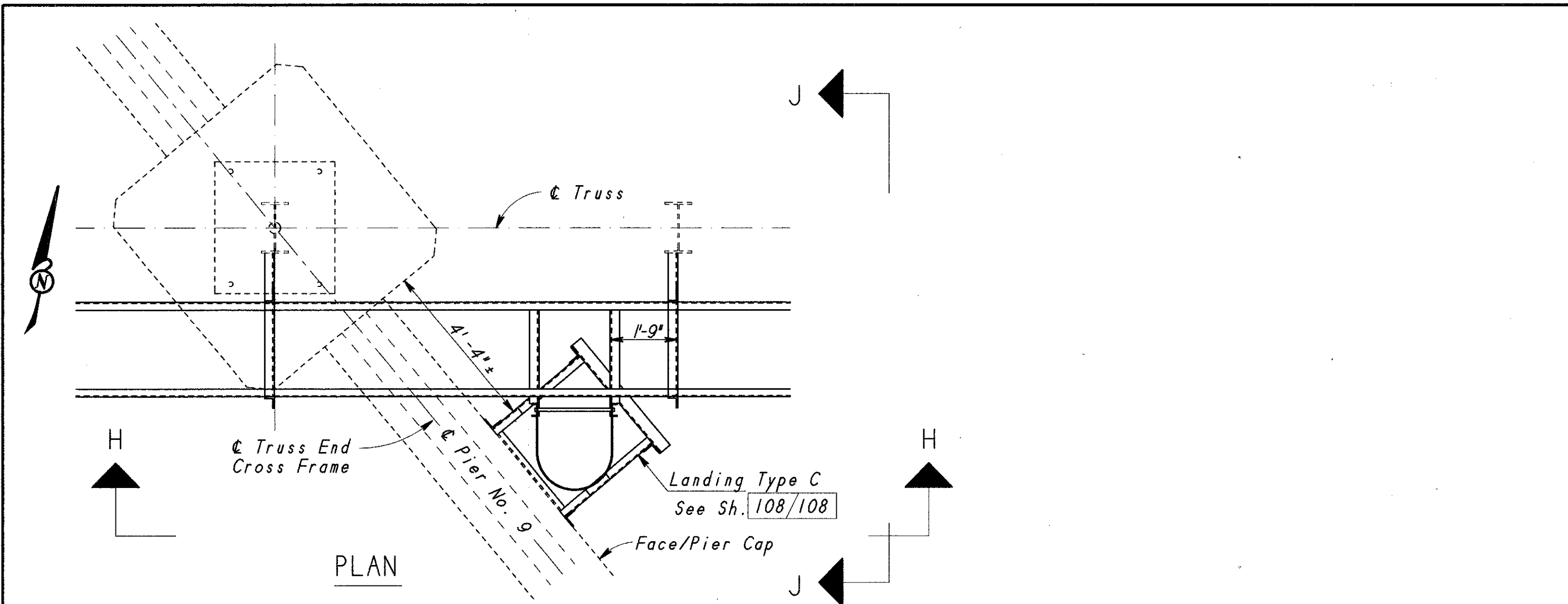
WALNUT 5344 GCD 2.666

F.H.W.A. REGION	STATE	PROJECT
5	OHIO	

180
182

HAM-471-00.25

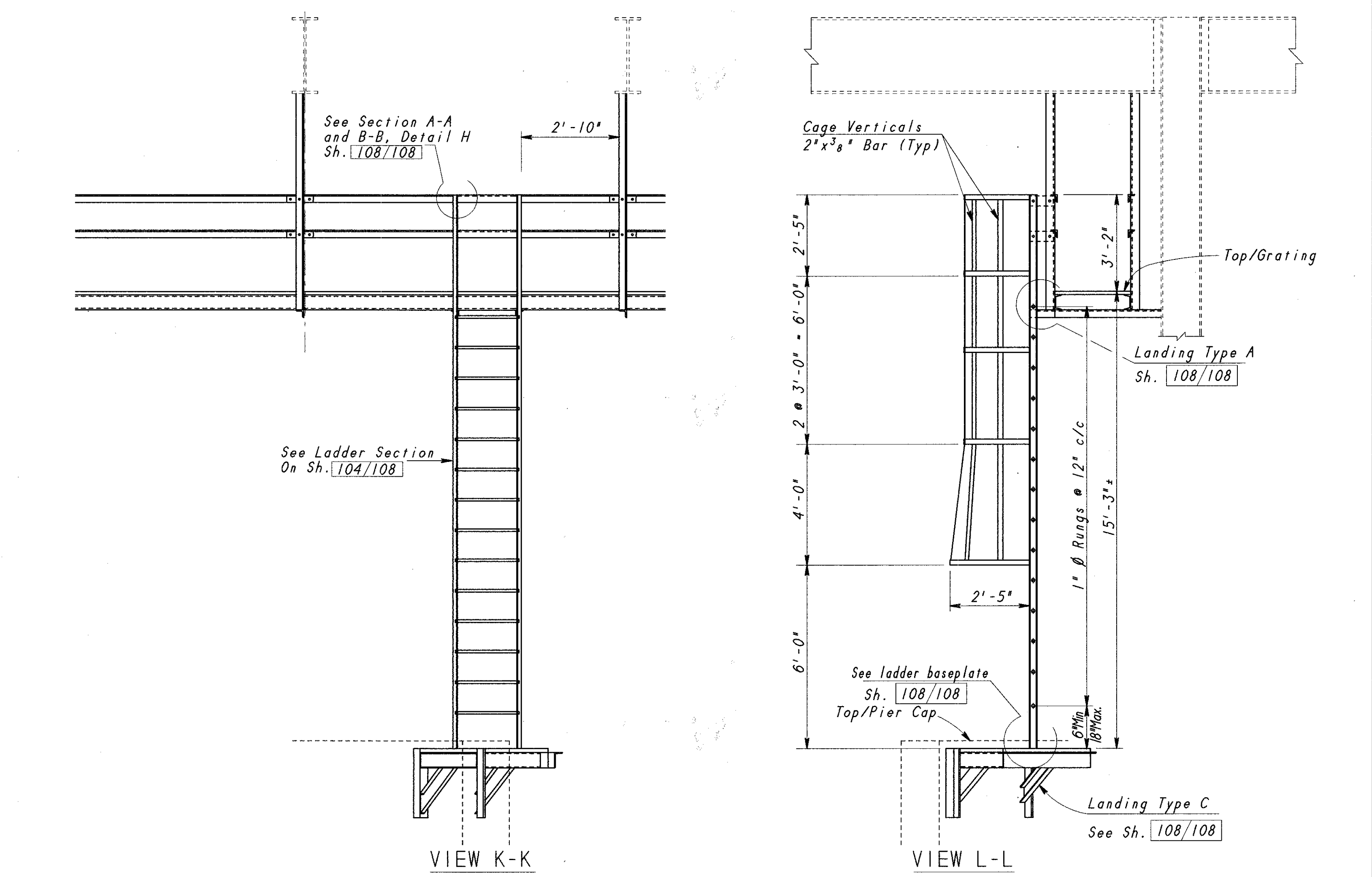
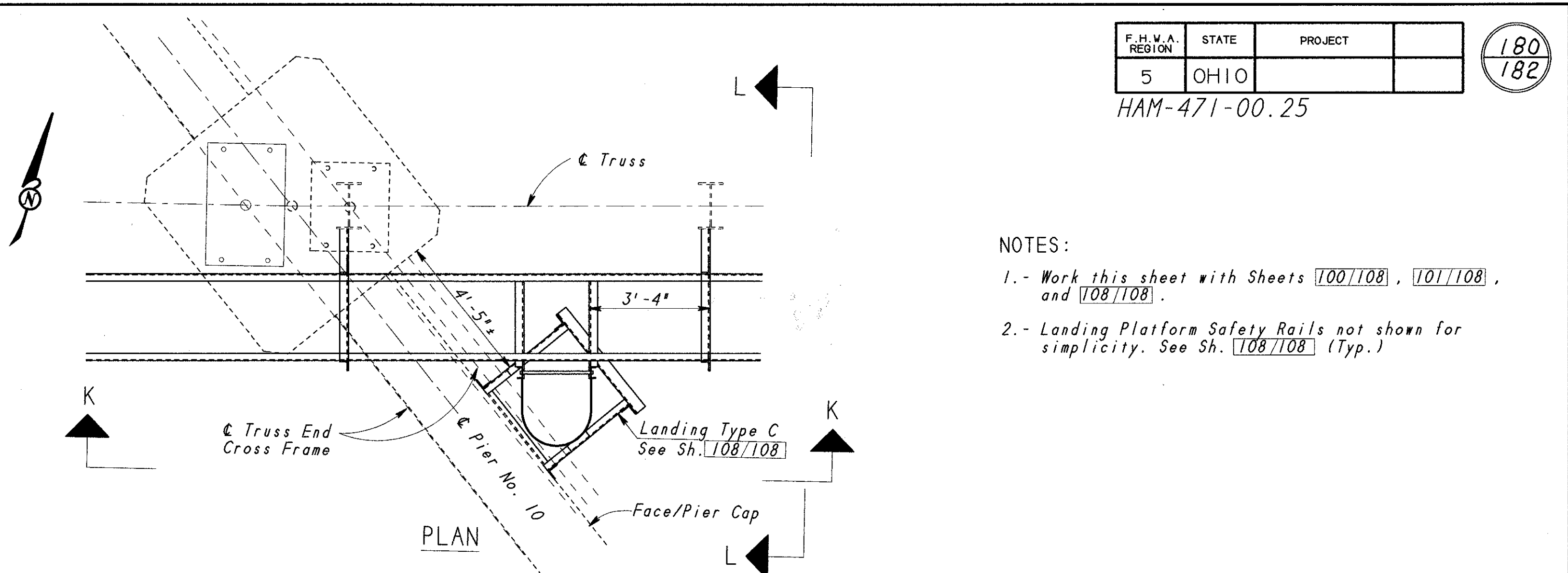
- NOTES:
- 1.- Work this sheet with Sheets [100/108], [101/108], and [108/108].
 - 2.- Landing Platform Safety Rails not shown for simplicity. See Sh. [108/108] (Typ.)



LEGEND

Existing Structure

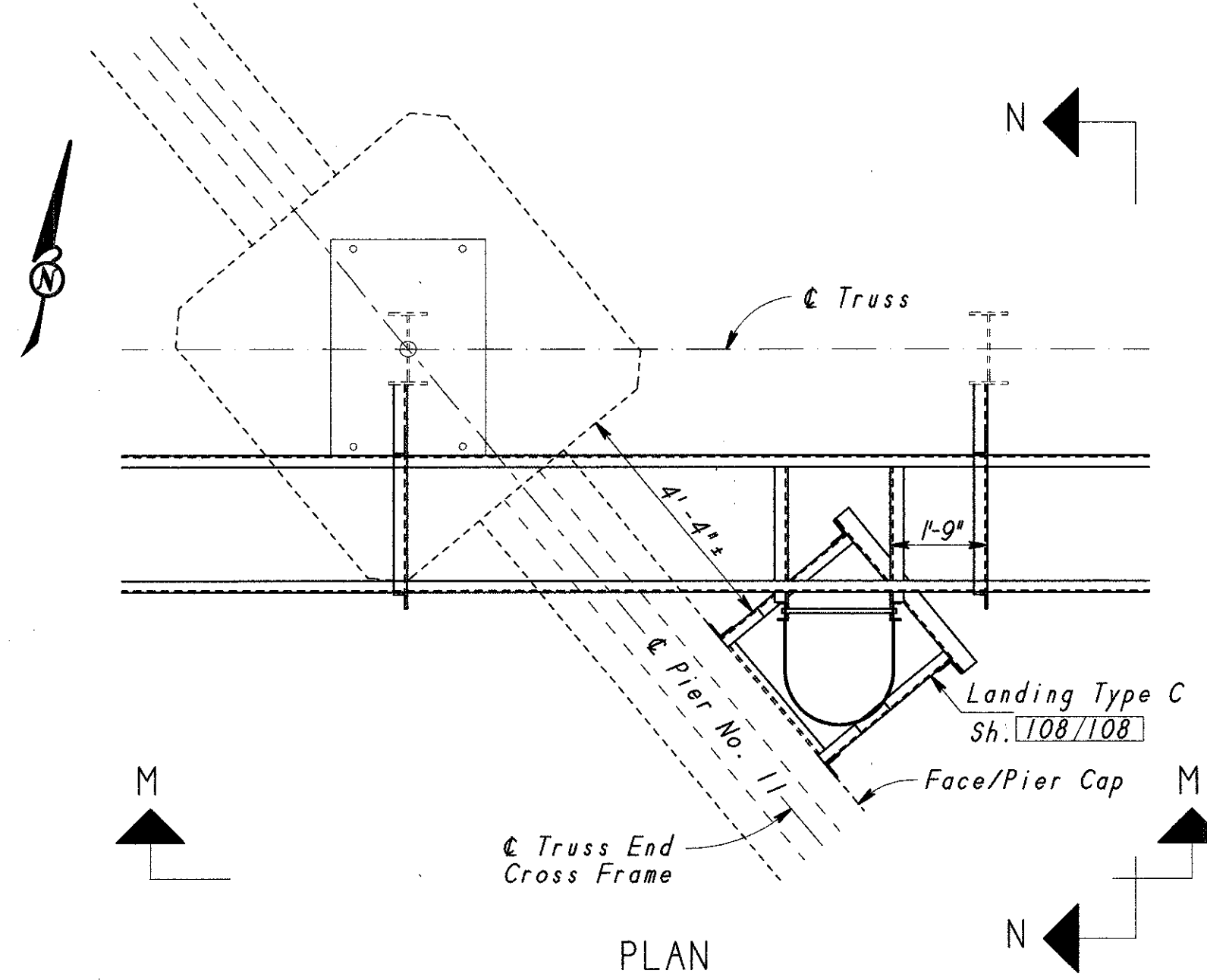
New Work



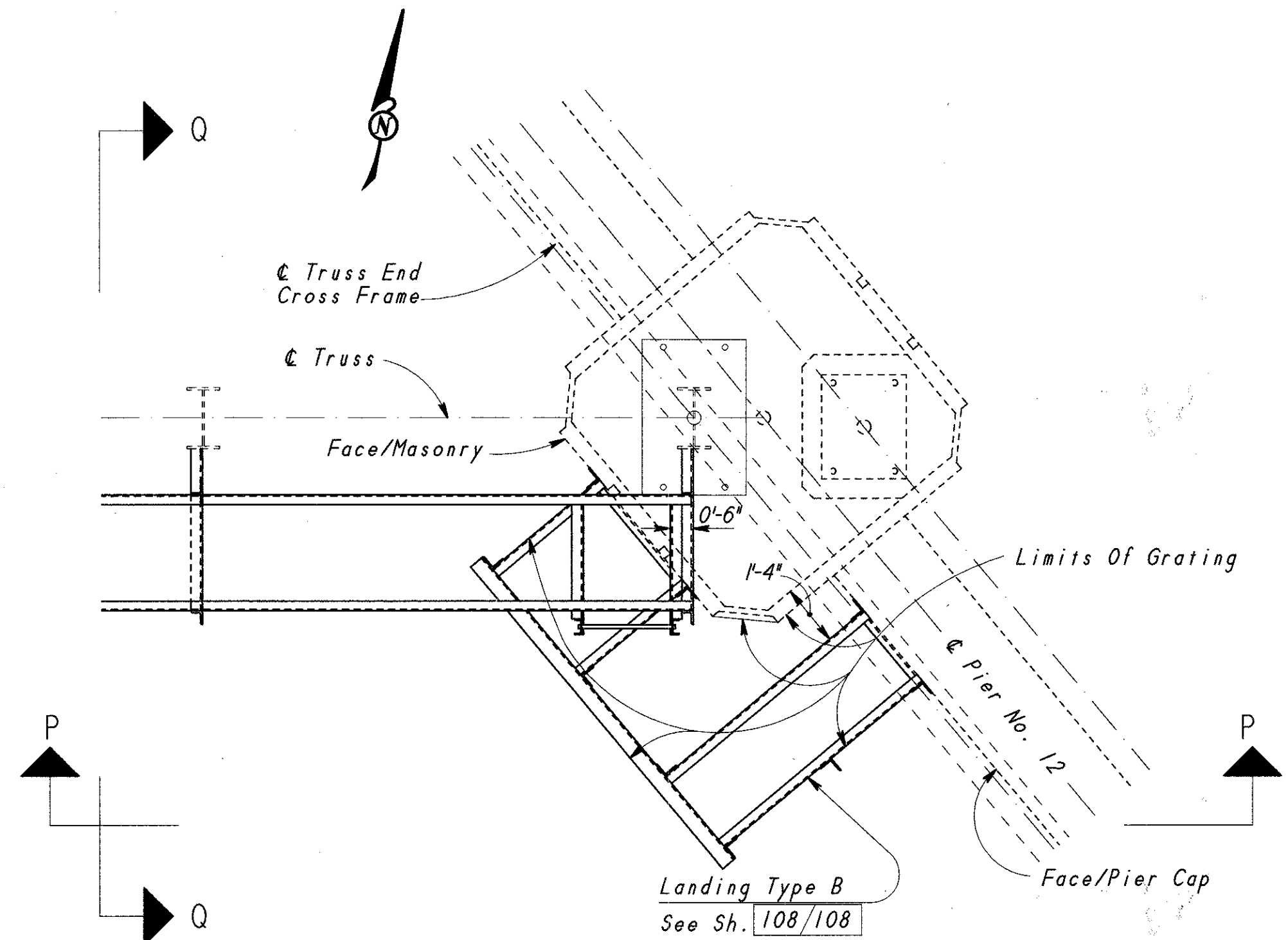
SERVICE LADDER NO. 6

		106/108
CATWALK SERVICE LADDER No. 5 (PIER NO. 9) No. 6 (PIER NO. 10) BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY DESIGNED: ED DRAWN: PLF TRACED: WDD CHECKED: IEH REVIEWED: April 96 DATE:		Sta. 30+55.40 Sta. 47+16.19 OHIO REVISIONS:

L-5546 Scale 2.6668

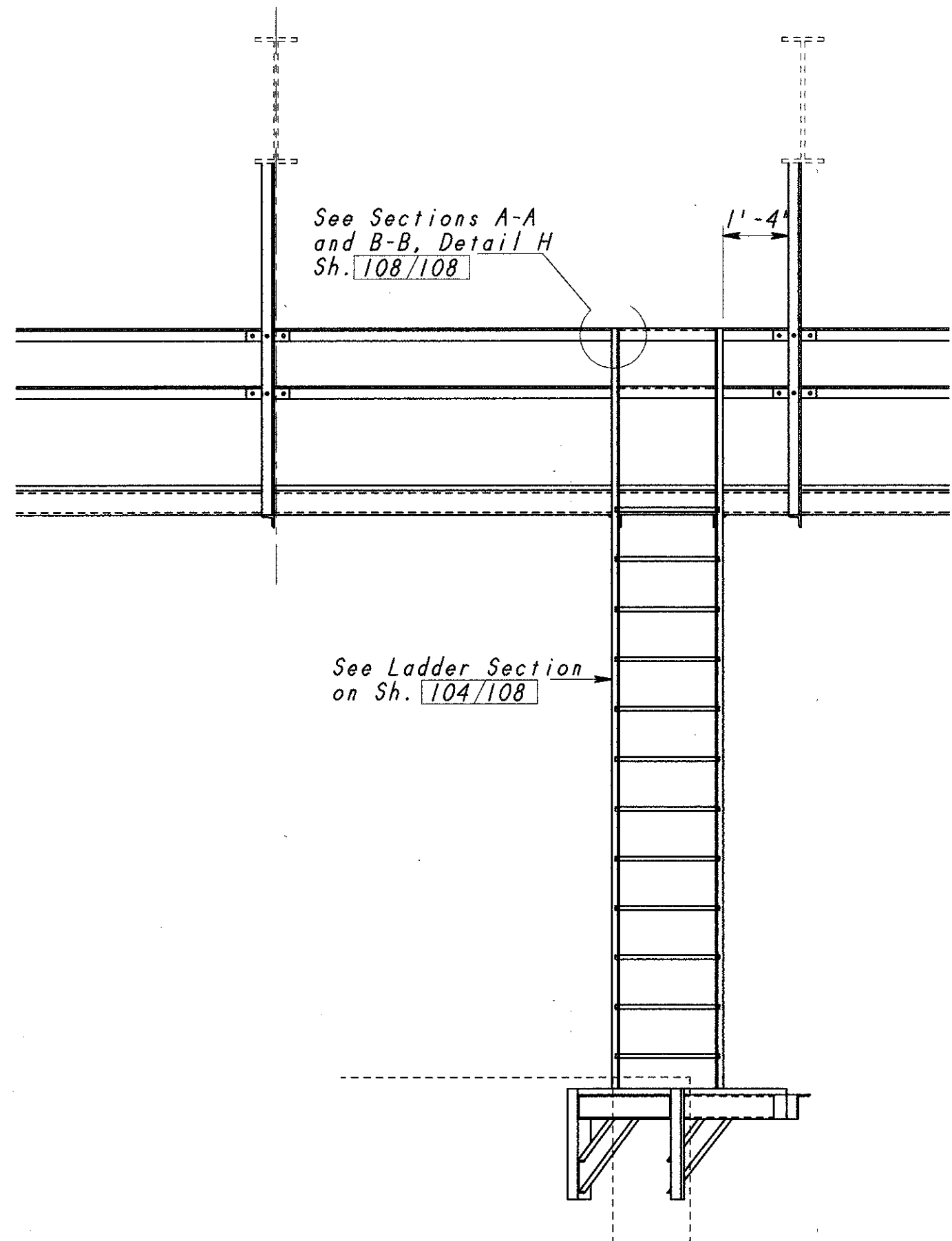


PLAN

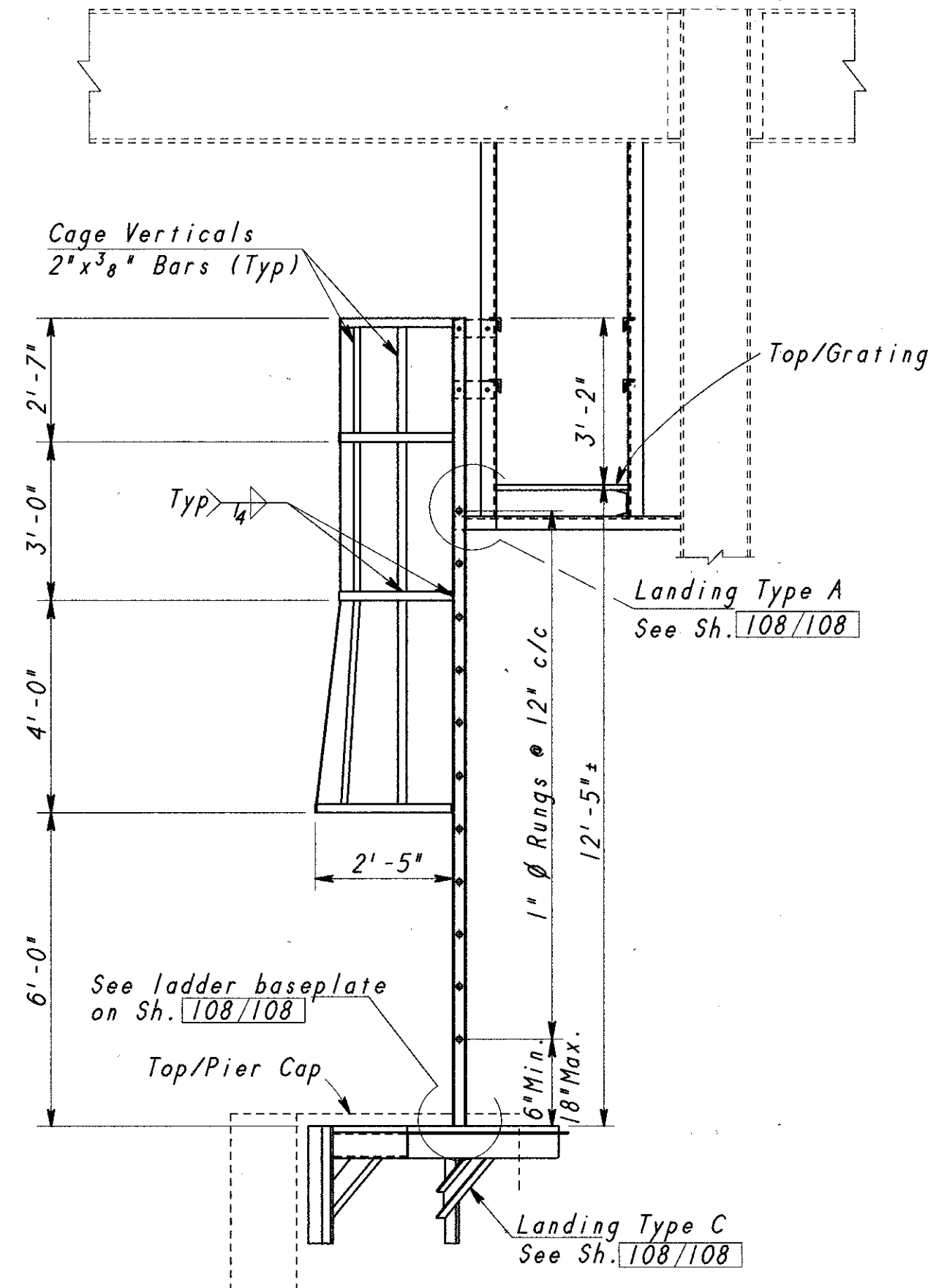


PLAN

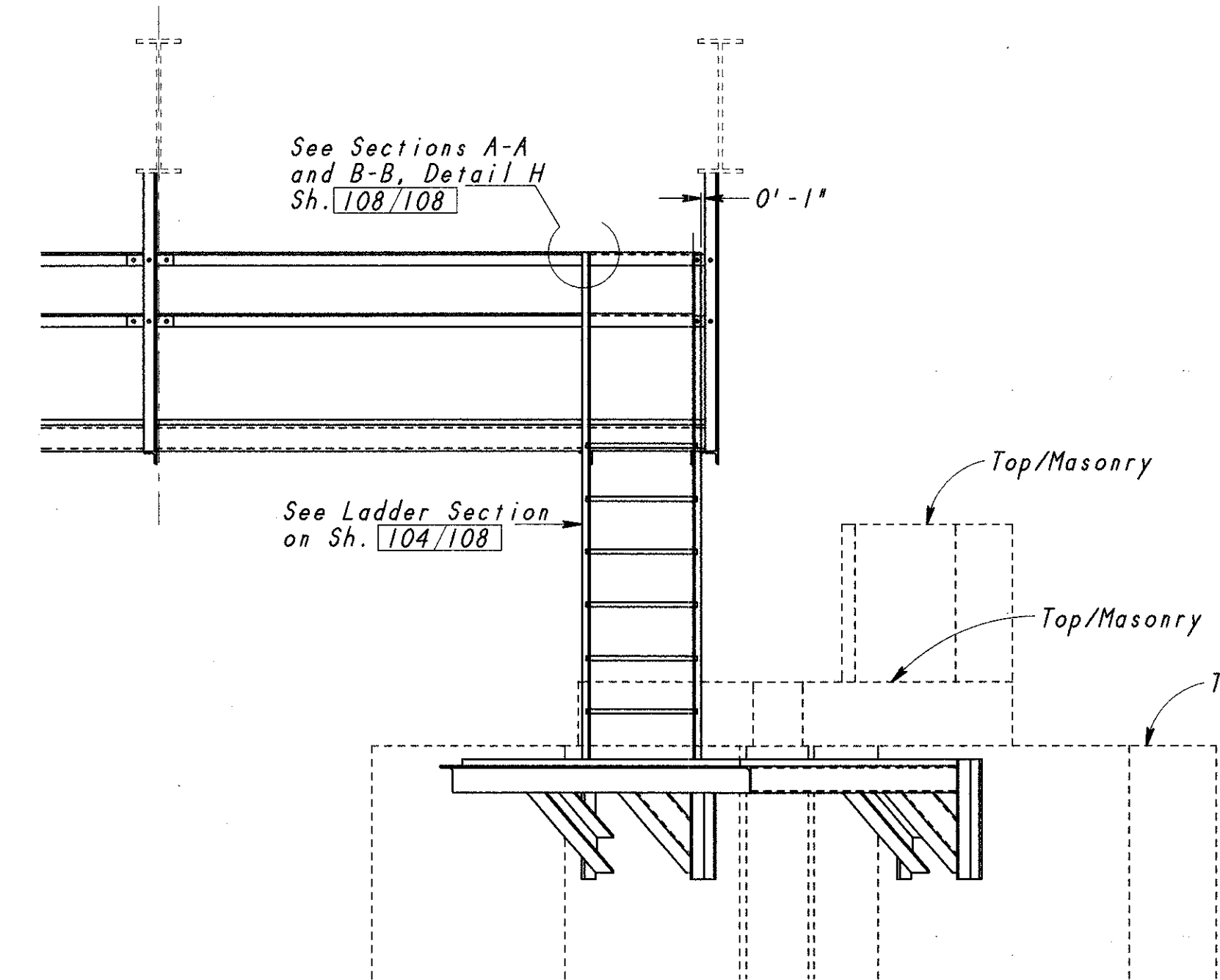
- NOTES:
- 1.- Work this sheet with Sheets [100/108], [101/108], and [108/108].
 - 2.- Landing Platform Safety Rails not shown for simplicity. See Sh. [108/108] (Typ.)



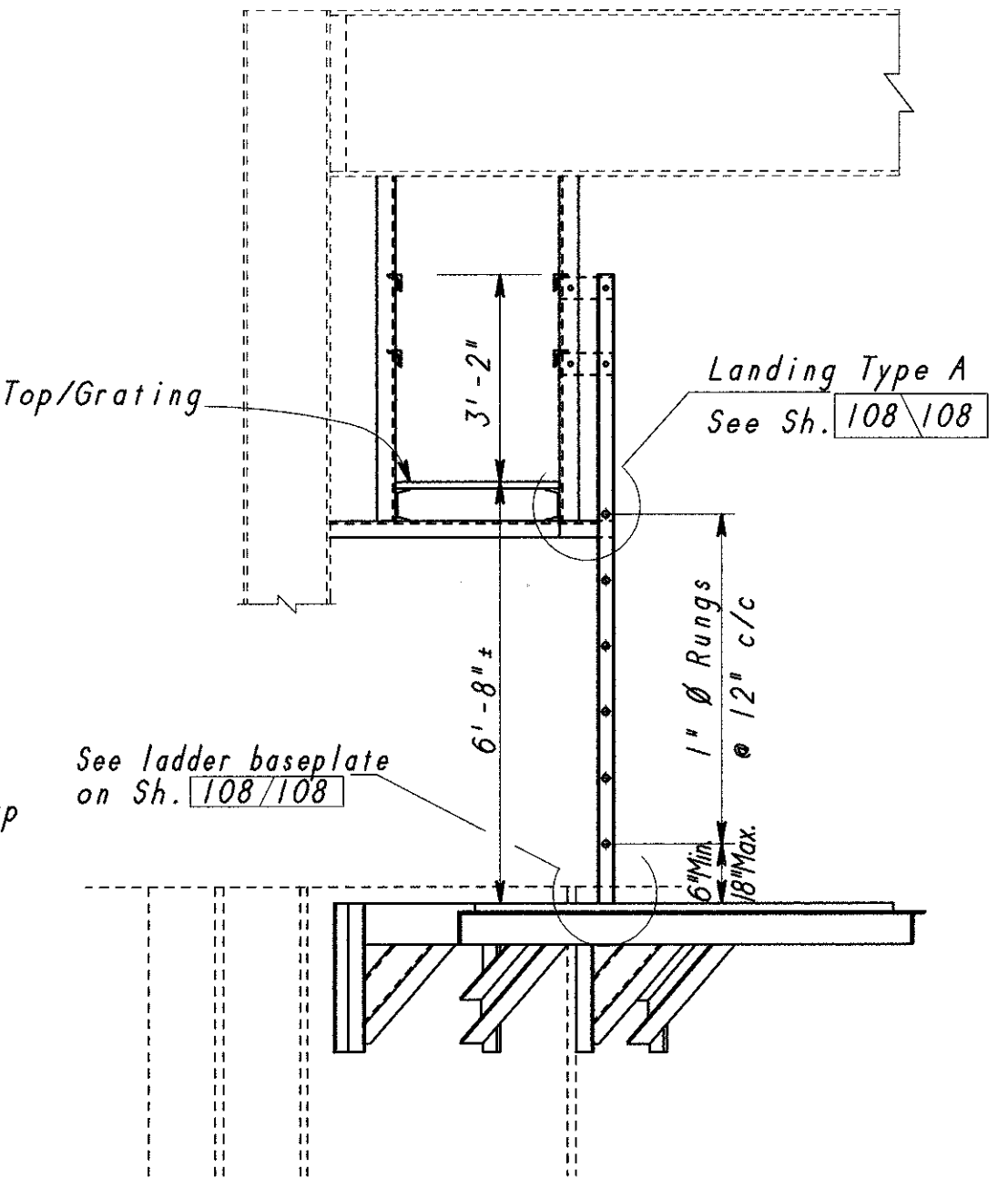
VIEW M-M



VIEW N-N



VIEW P-P



VIEW Q-Q

SERVICE LADDER NO. 8

Note: Service Ladder No. 8 Does Not Require Safety Hoops.

LEGEND

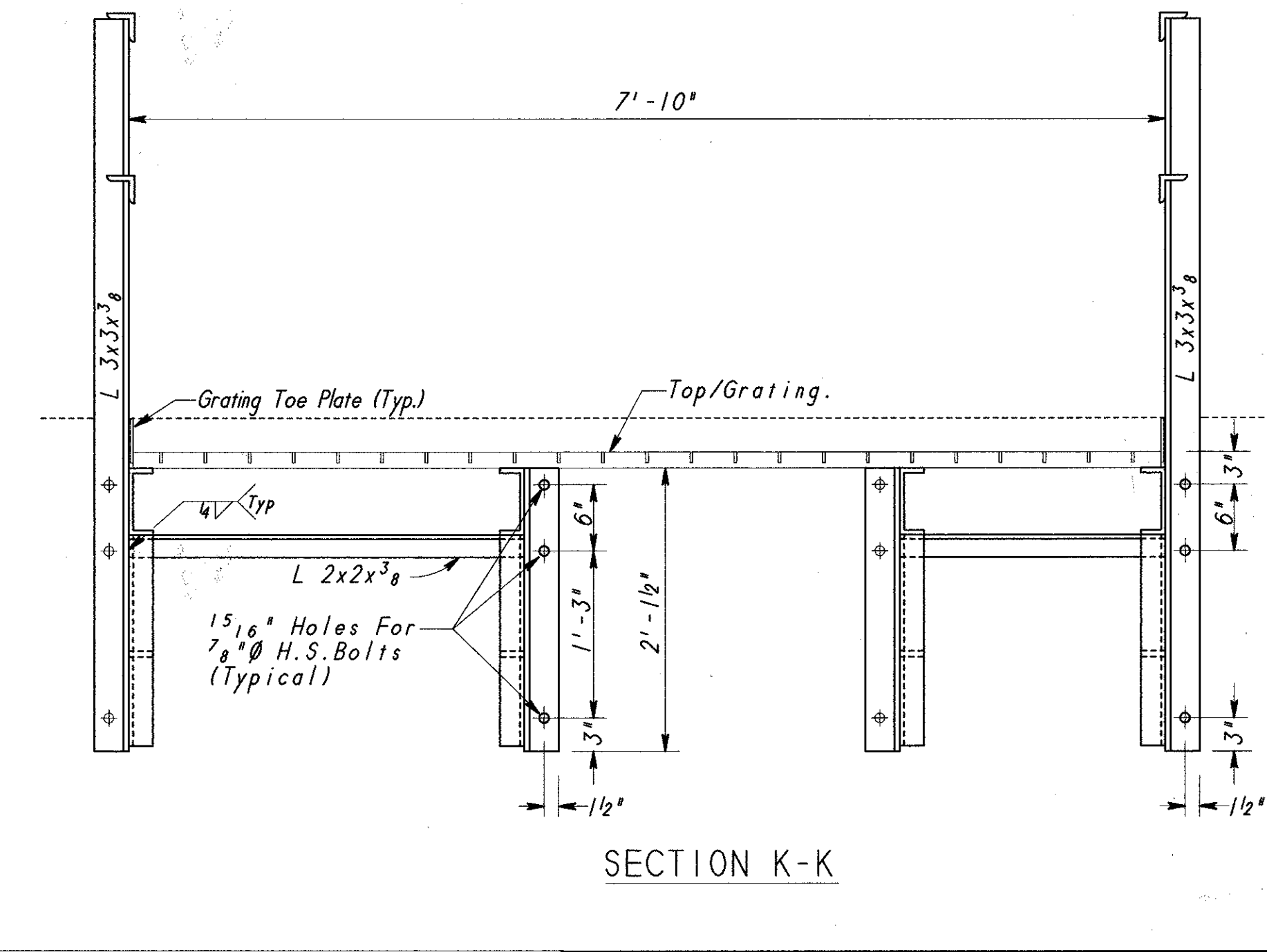
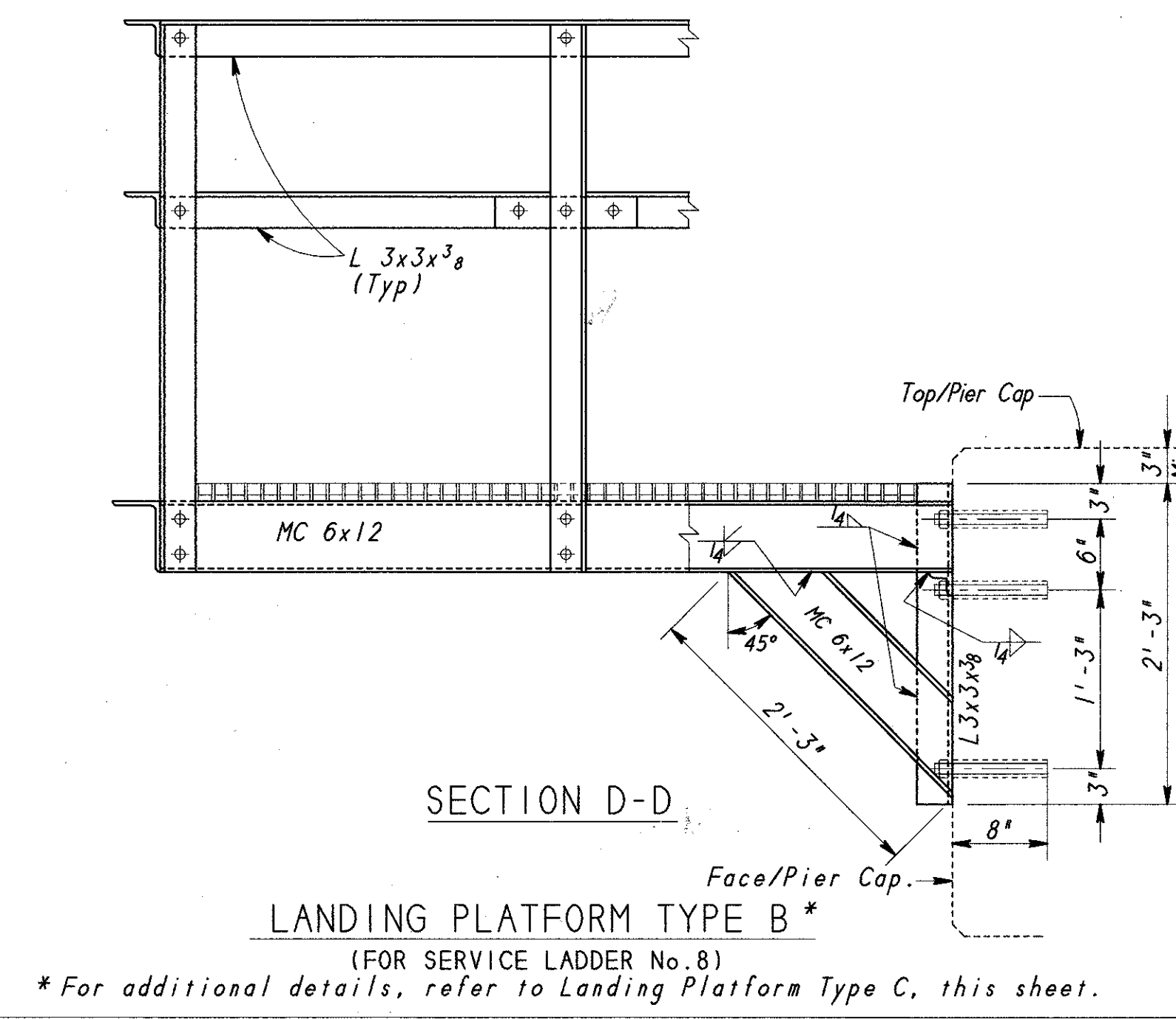
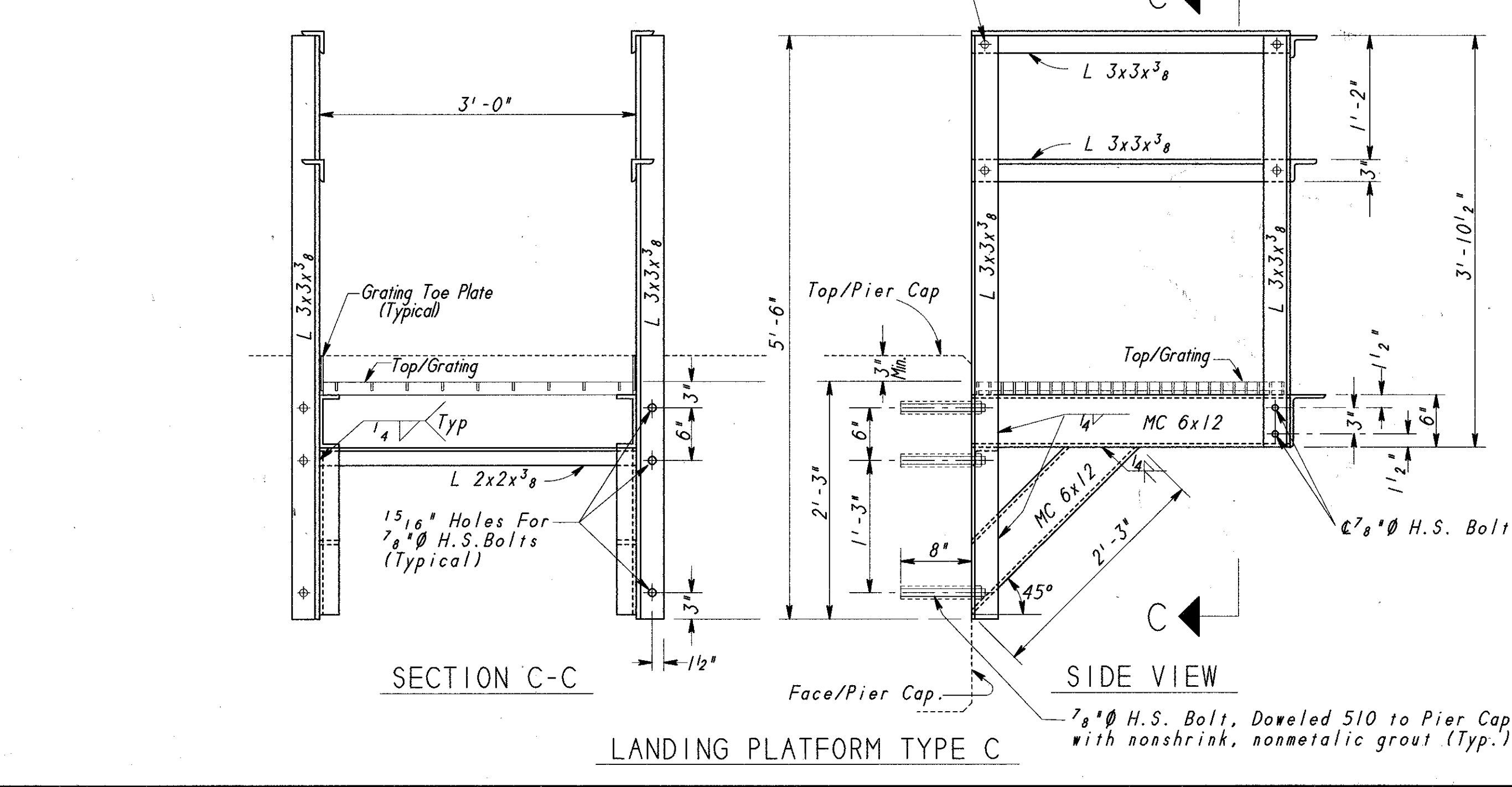
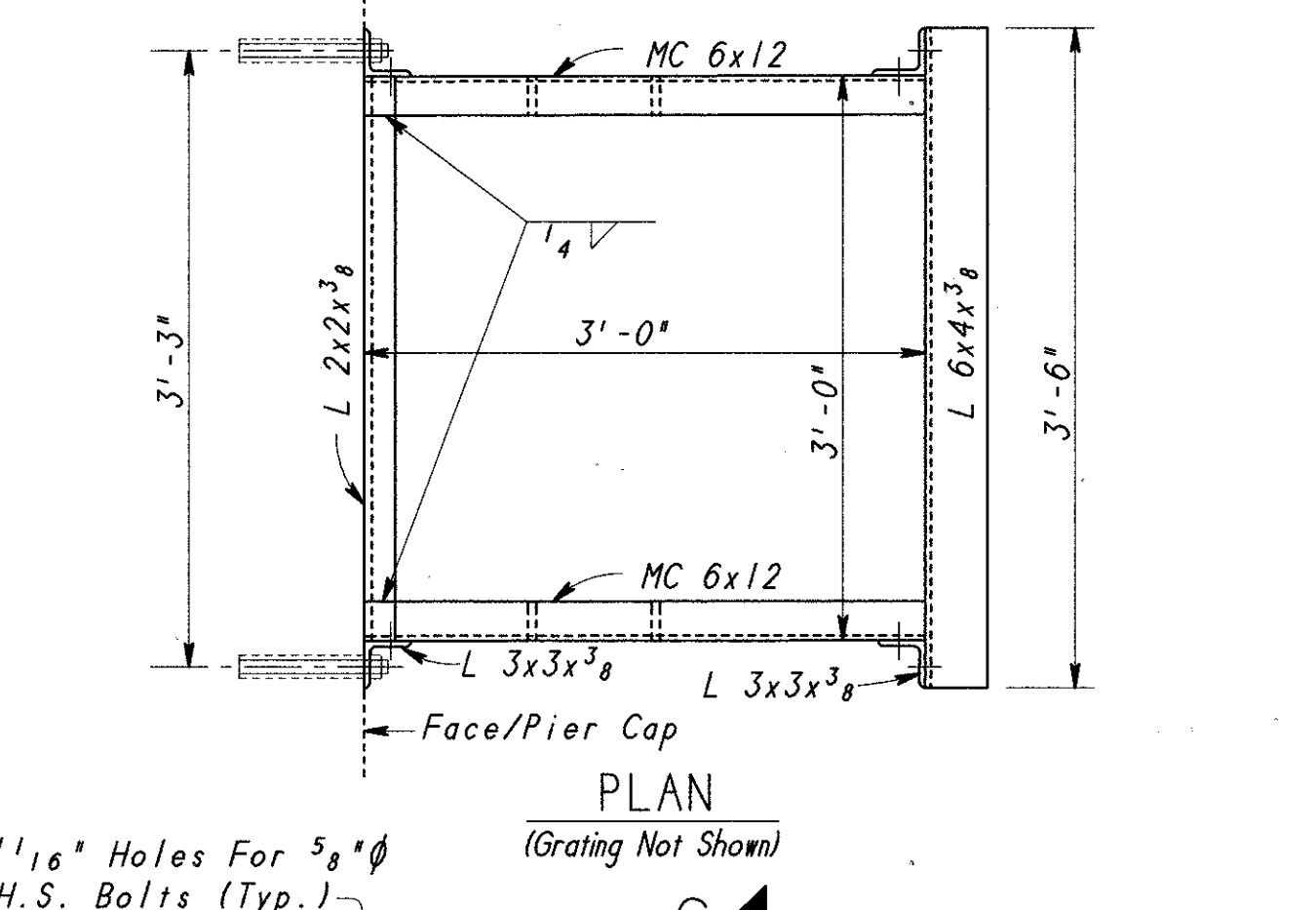
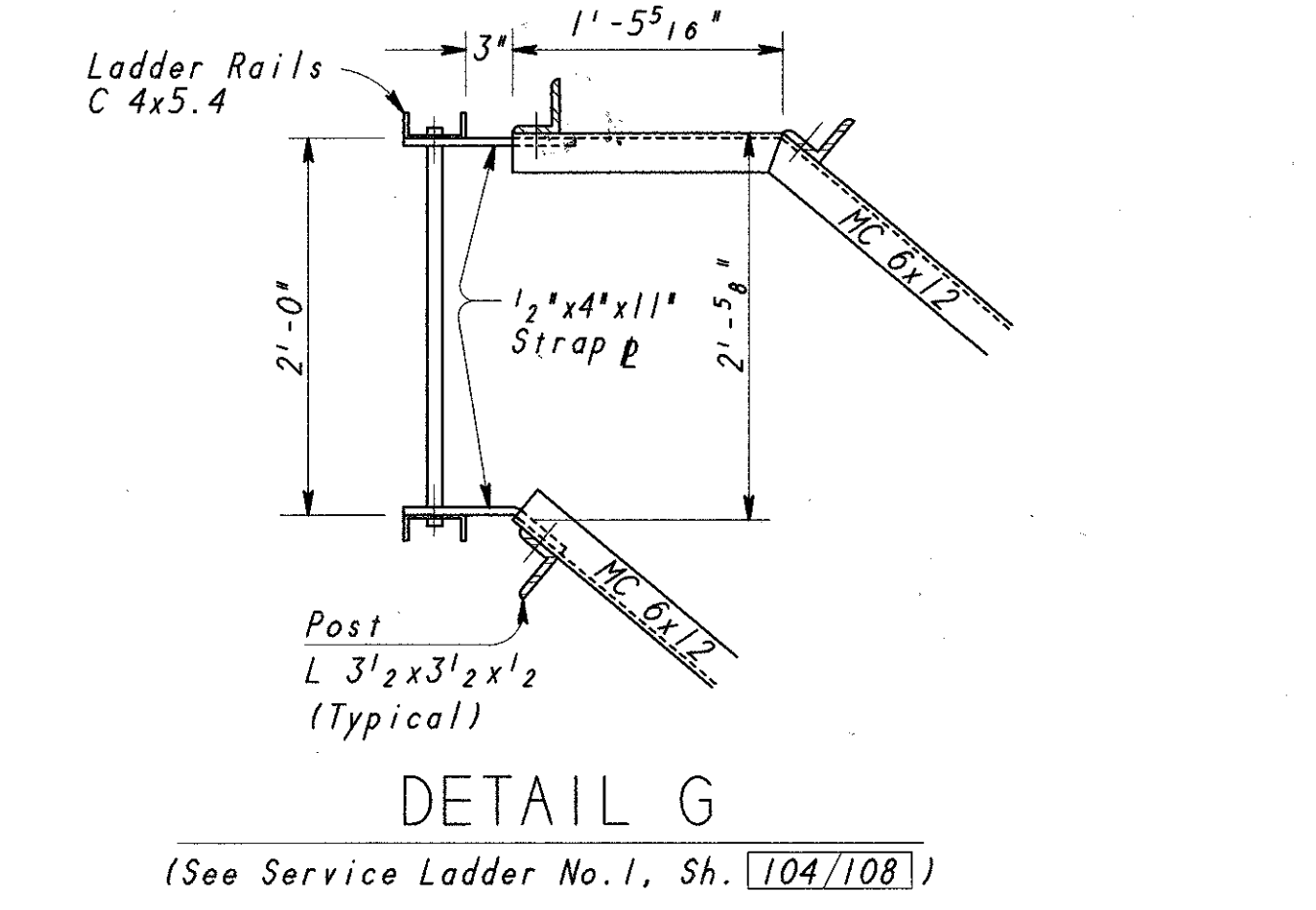
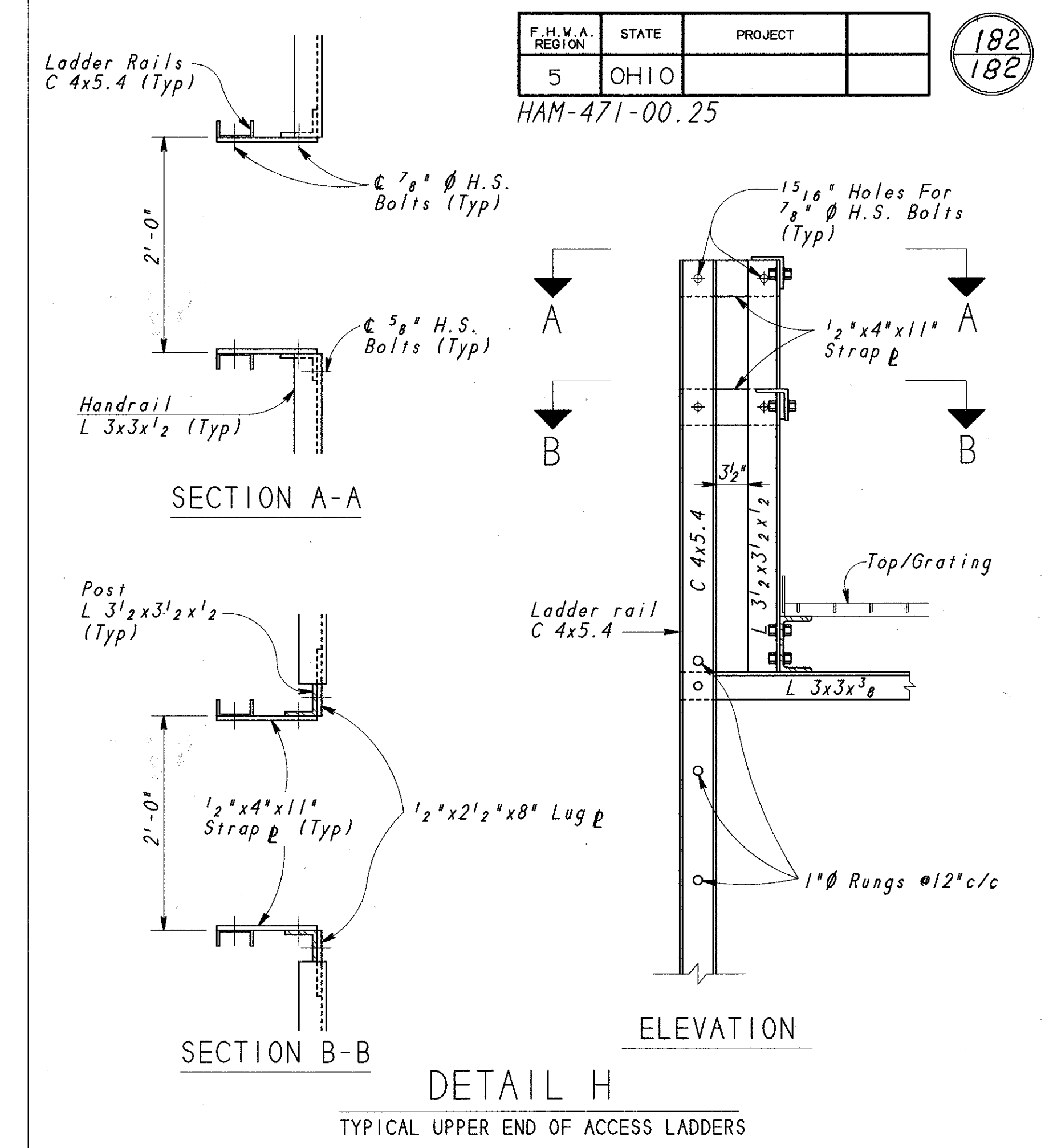
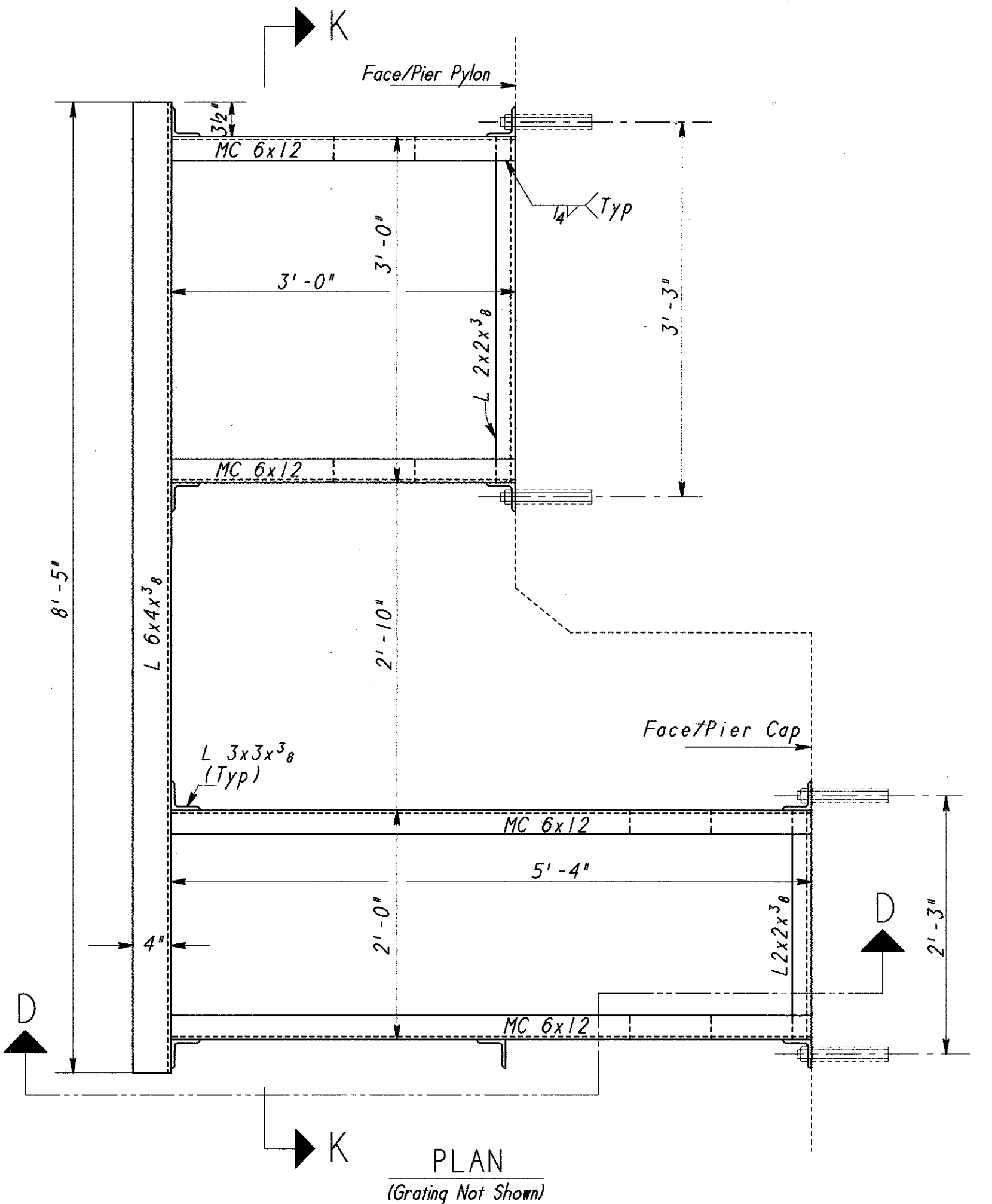
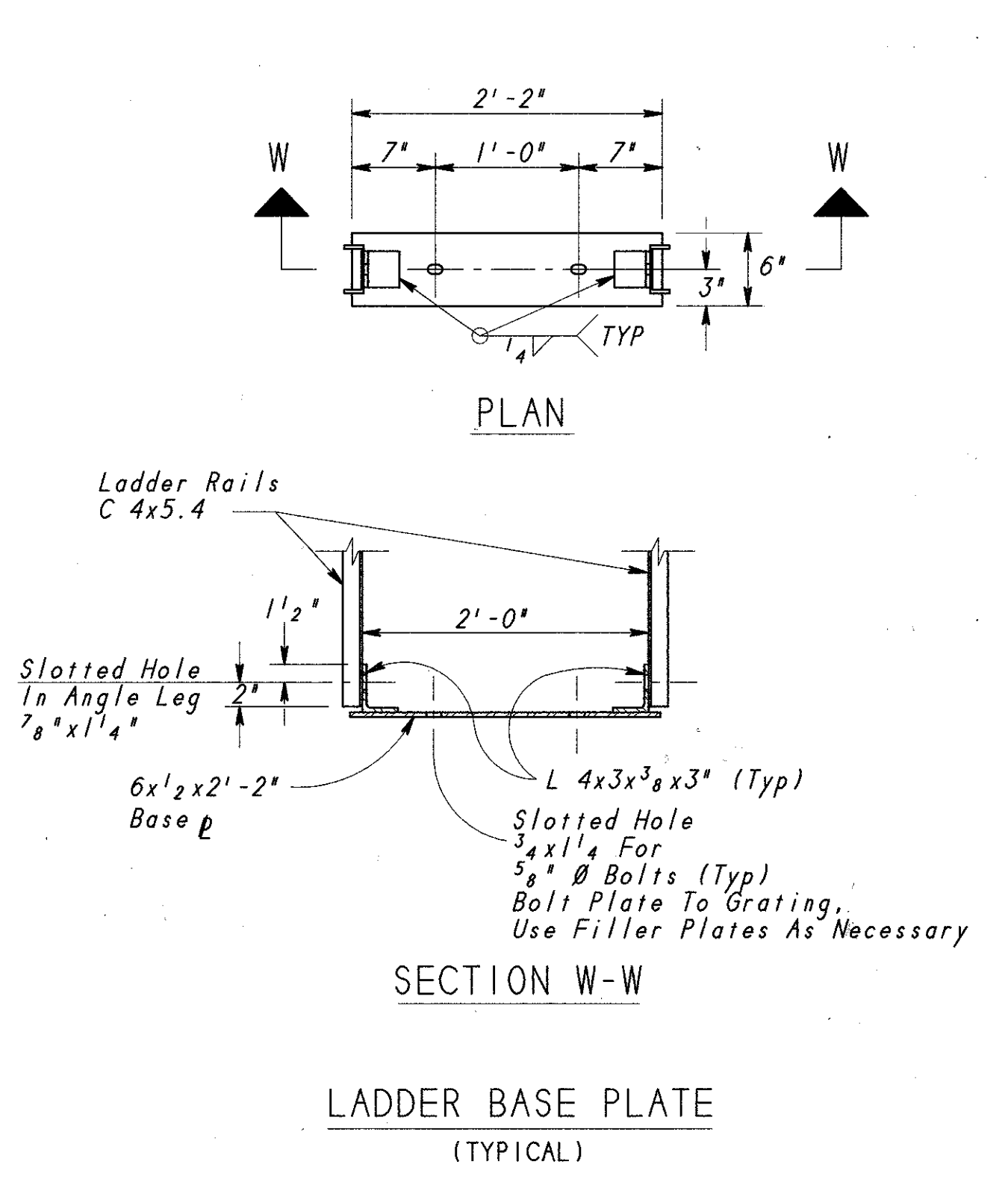
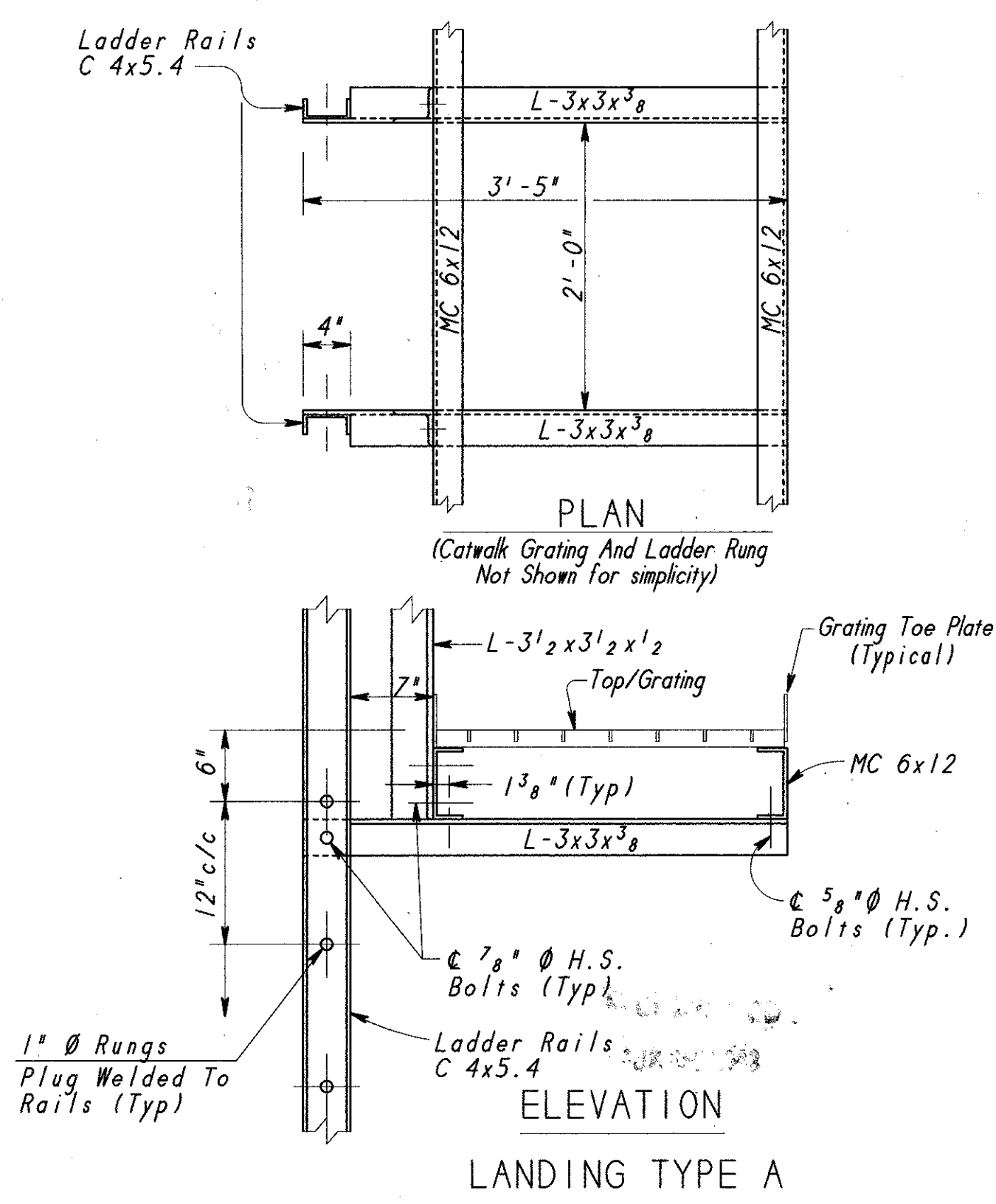
	Existing Structure
	New Work

SERVICE LADDER NO. 7

Pflum, Klausmeyer & Gehrum Consultants		107/108
CATWALK SERVICE LADDER		
No. 7 (PIER NO. 11)		
No. 8 (PIER NO. 12)		
BRIDGE No. HAM-471-0025		
(COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40
DESIGNED	DRAWN	TRACED
ED	PLF	WDD
CHECKED	REVIEWED	DATE
WDD	IEH	April 96
REVISED		

WALK/SL718 GCD SCALE 2:6666

HAM-471-00.25



* For additional details, refer to Landing Platform Type C, this sheet.

- NOTES:**
1. - Work this sheet with sheets 104/108 thru 107/108.
 2. - The Contractor shall field verify the landing platform configuration, and incorporate the proper detail with the catwalk shop drawings for approval by the Engineer (Typical).
 3. - For bolt details which are not shown on this sheet, refer to CATWALK DETAILS, Sh. 102/108 and 103/108.
 4. - See description of Item 513, STRUCTURAL STEEL, MISC.: CATWALK INCLUDING LADDERS, and Item 513, CATWALK GRATING, on Sheets 101/108, and 102/108 respectively.

Pflum, Klausmeier & Gehrum Consultants		108/108
CATWALK SERVICE LADDER DETAILS		
BRIDGE No. HAM-471-0025 (COLUMBIA PARKWAY VIADUCT OVER I-471)		
HAMILTON COUNTY		Sta. 30+55.40 Sta. 47+16.19
DESIGNED ED	DRAWN PLF	TRACED WDD
CHECKED WDD	REVIEWED IEH	DATE April 96