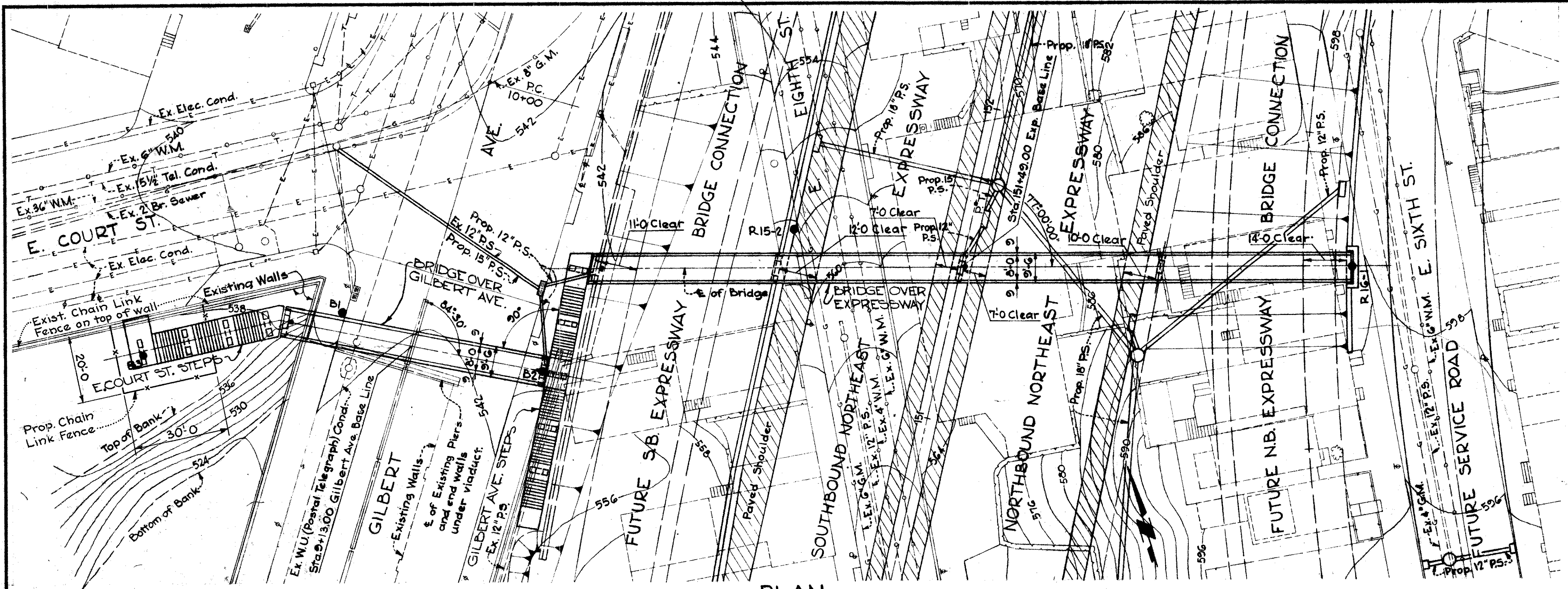
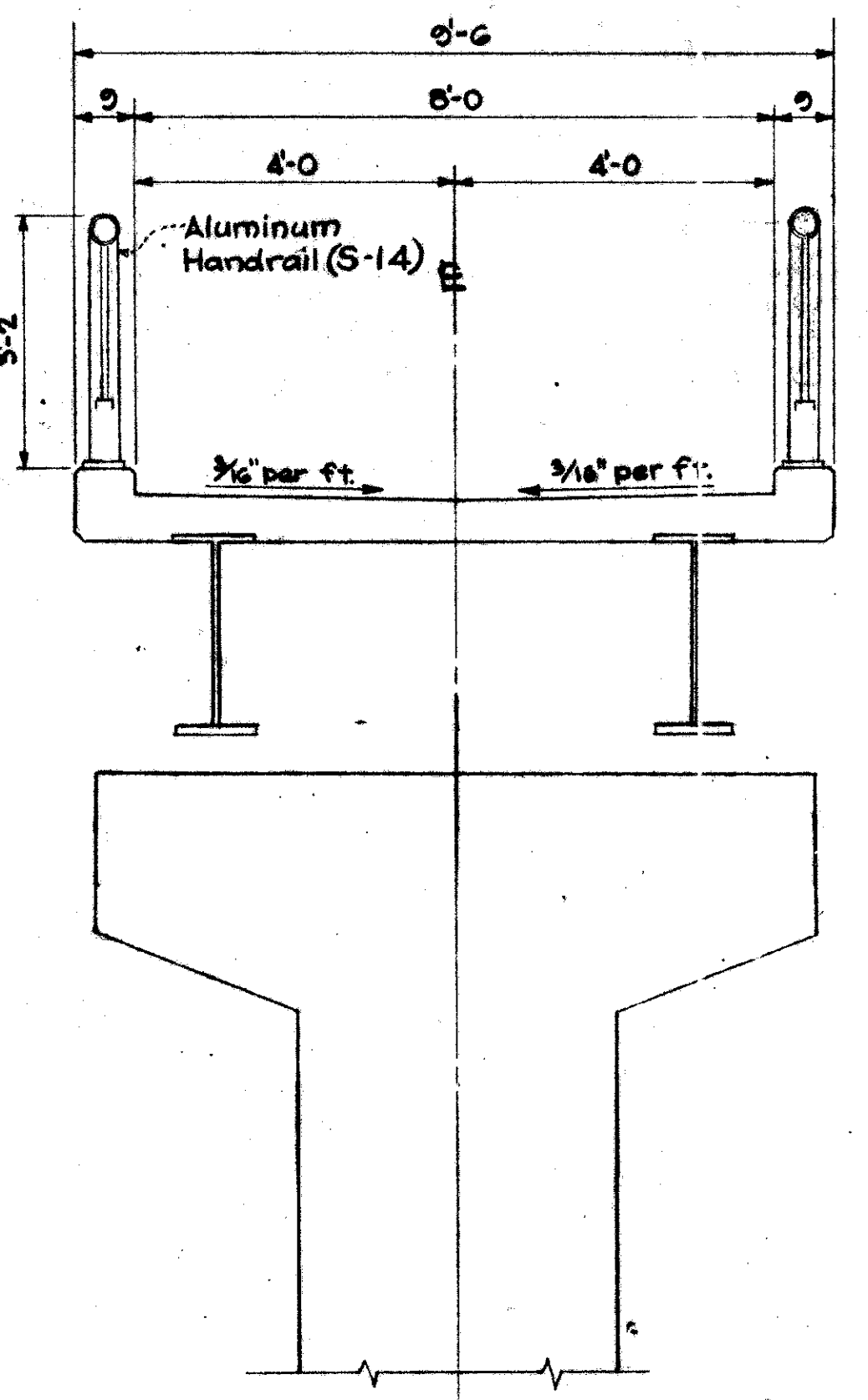


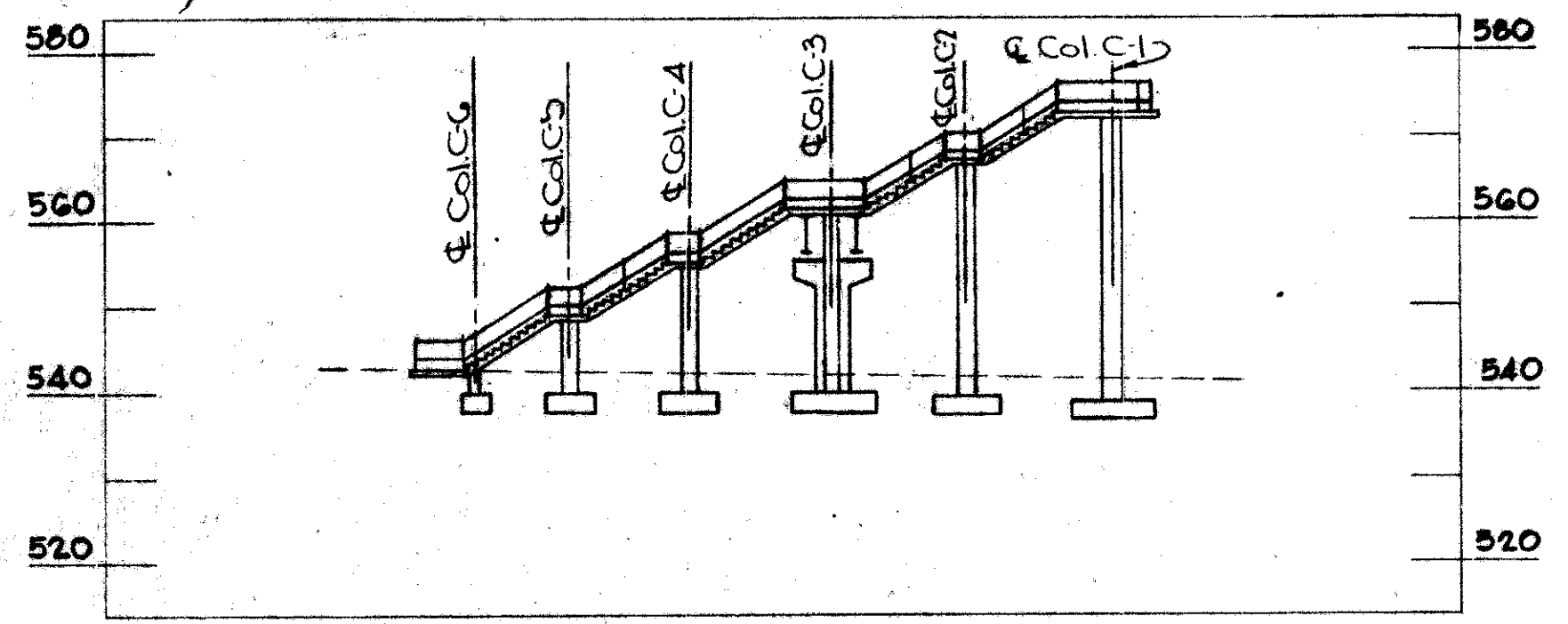
HAMILTON COUNTY
HAM-71-(15G)(2.51)



PLAN

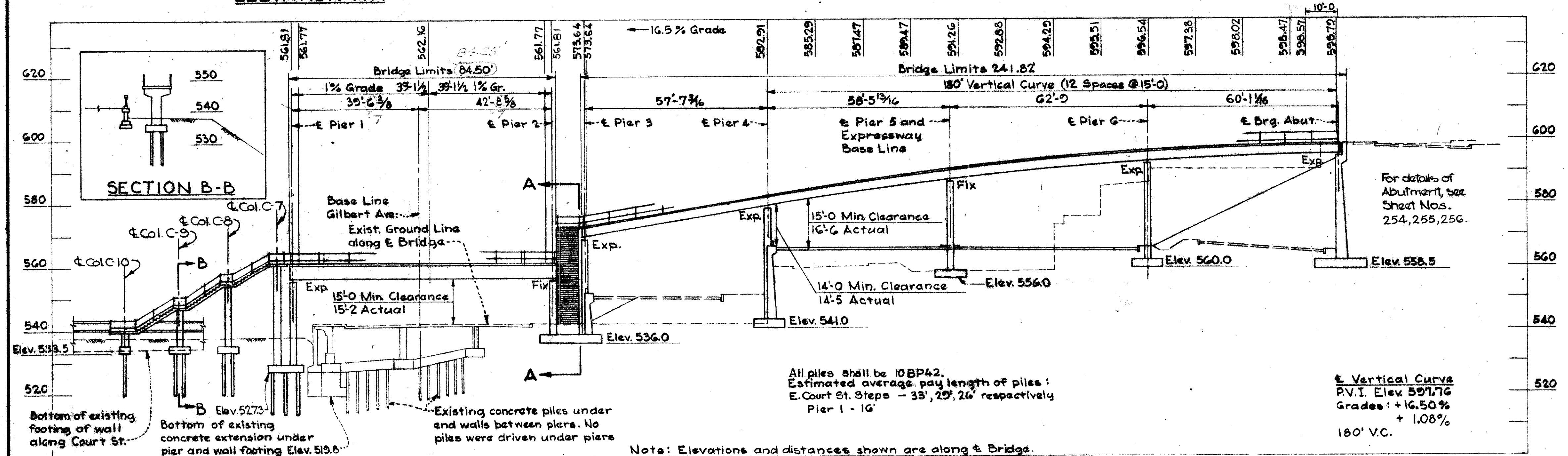


SECTION THRU BRIDGE



ELEVATION A-A

● Symbol denotes Test Boring.
For logs of borings see Sheet No. 7A.



ELEVATION

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PROPOSED STRUCTURE

TYPE: 4 Span Continuous Rolled Beam with reinforced concrete deck and substructure. Single Span Plate Girder with reinforced concrete deck and substructure.

SPANS: 56.73', 58.48', 62.75', 61.11' (Rolled Beam Structure) 82.25' (Plate Girder Structure).

WIDTH: 8'-0" Clear between curbs.

LOADING: 85 lb./sq. ft. (uniform)

SKEW: As shown on Plan.

CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

SITE PLAN

BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.
Sta. 151+49 N.B. Main Line

CINCY BRIDGE NO. 7

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
L.E.N.	J.D.A.		W.W.	W.W.	3-8-65	8-27-66

Bottom of existing footing of wall along Court St. Elev. 533.5

Bottom of existing concrete extension under pier and wall footing Elev. 535.5

Existing concrete piles under and walls between piers. No piles were driven under piers.

All piles shall be 10BP42. Estimated average pay length of piles: E. Court St. Steps - 33', 29', 26' respectively. Pier 1 - 16'

Note: Elevations and distances shown are along & Bridge.

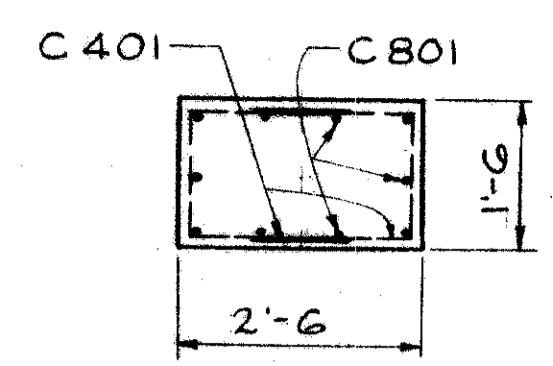
For details of Abutment, see Sheet Nos. 254, 255, 256.

Vertical Curve
P.V.I. Elev. 597.76
Grades: +16.50%
+ 1.08%
180' V.C.

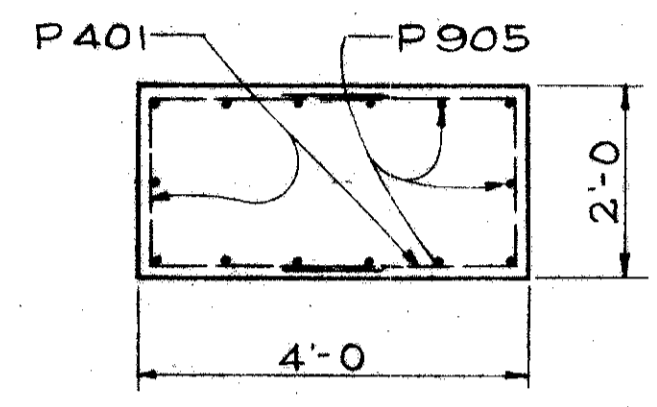
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		



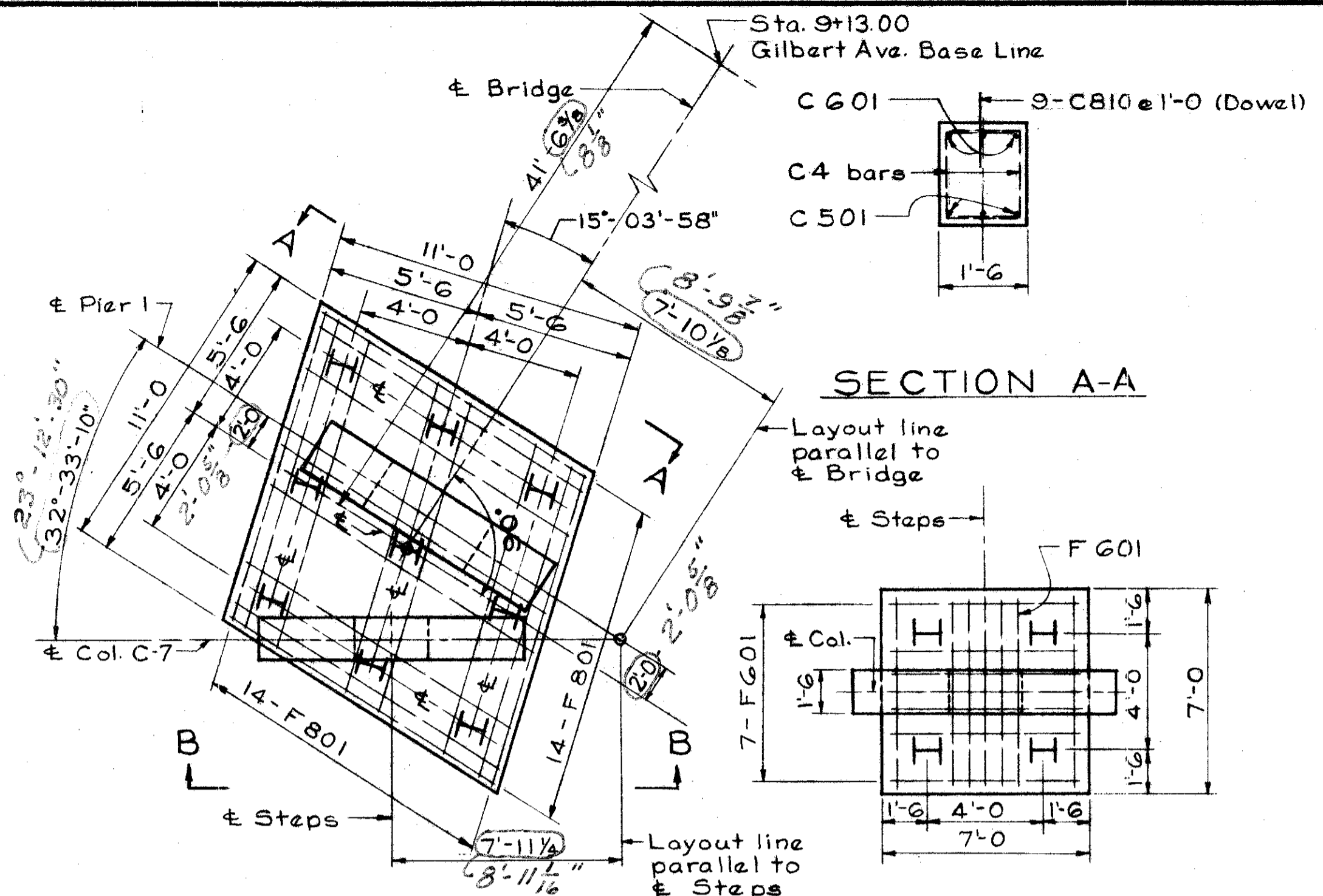
Typical for all plans this sheet.



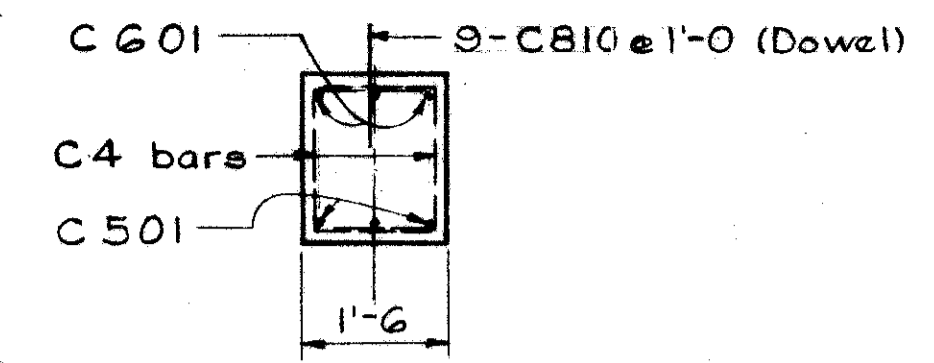
SECTION D-D



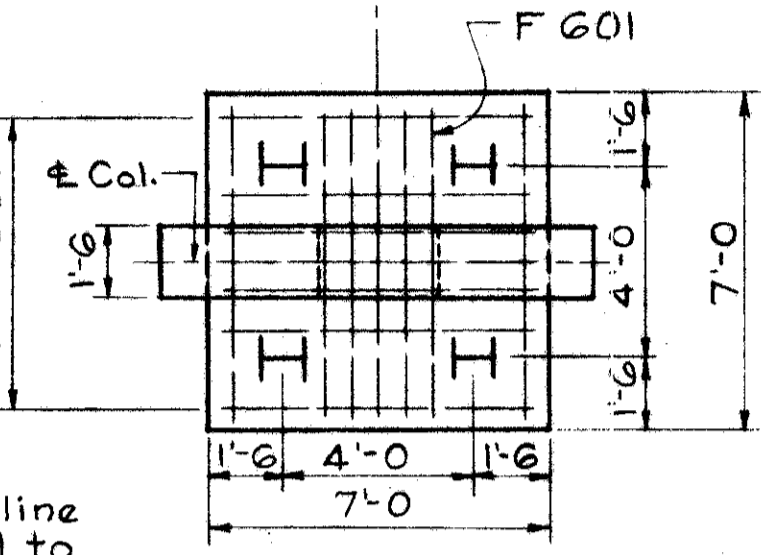
SECTION C-C



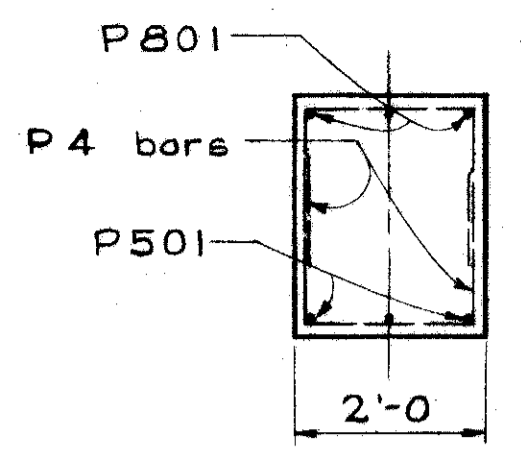
PLAN (Pier 1 & Column C-7)



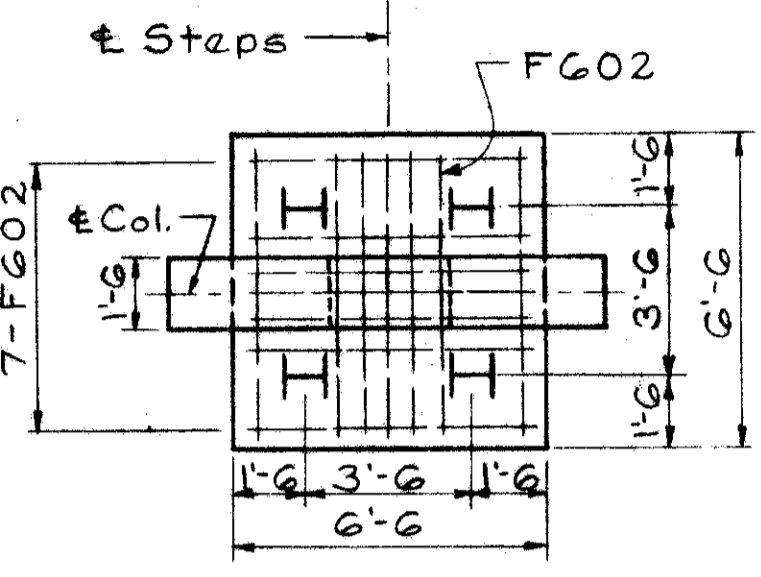
SECTION A-A



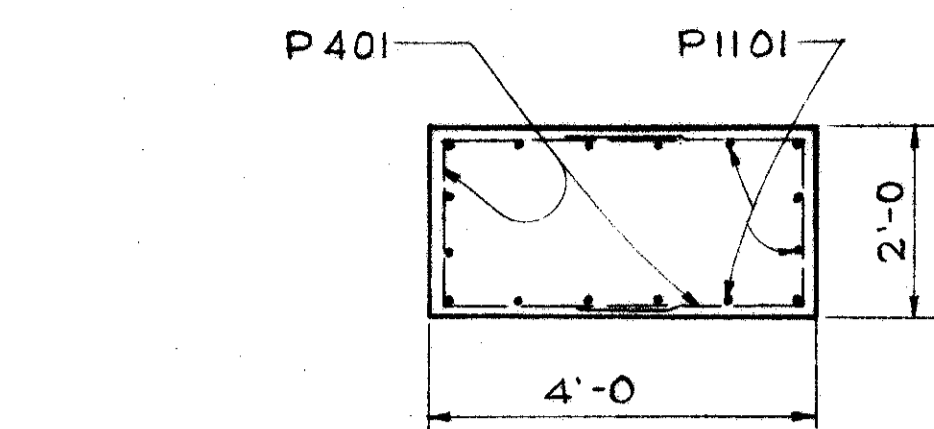
PLAN (Column C-8)



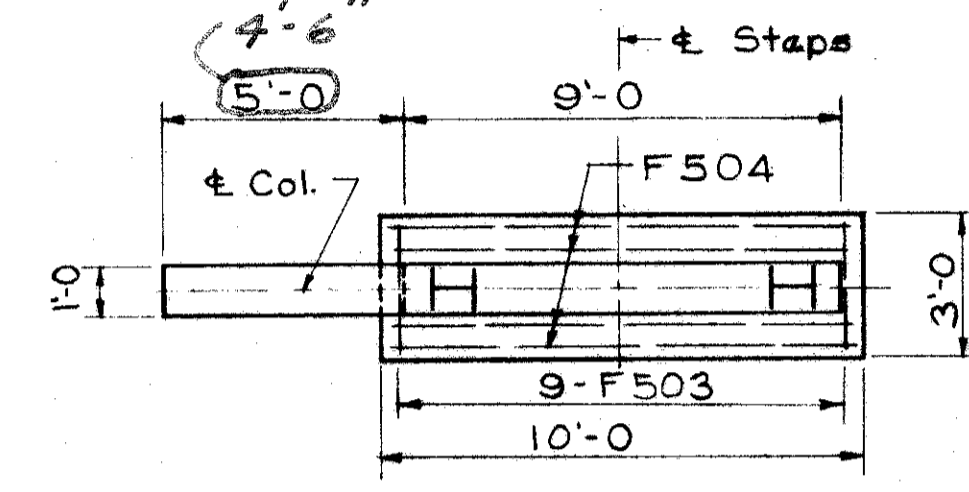
SECTION B-B



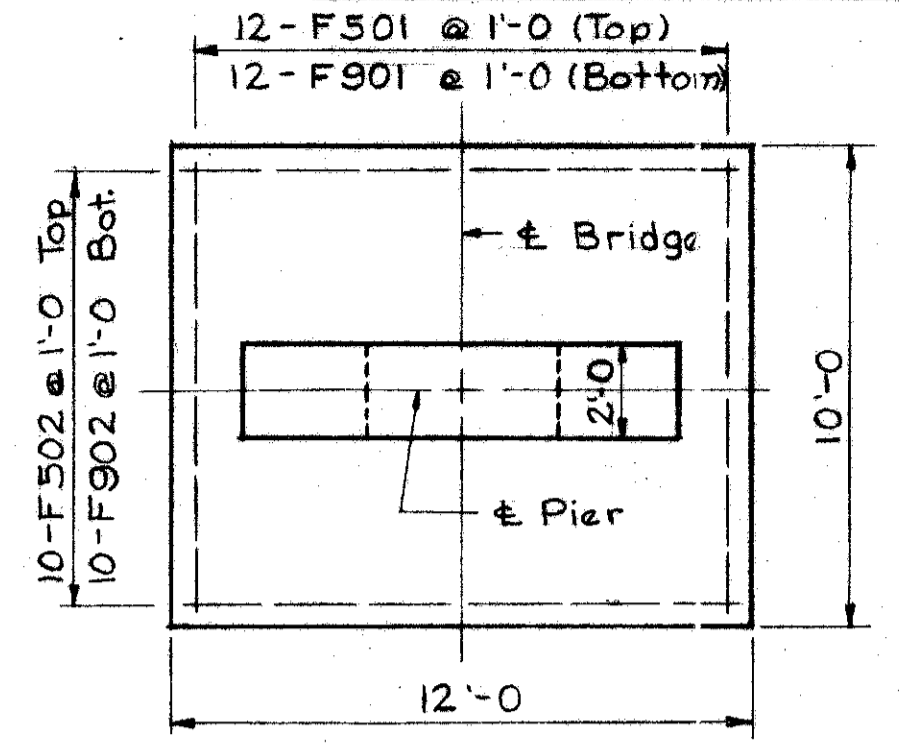
PLAN (Column C-9)



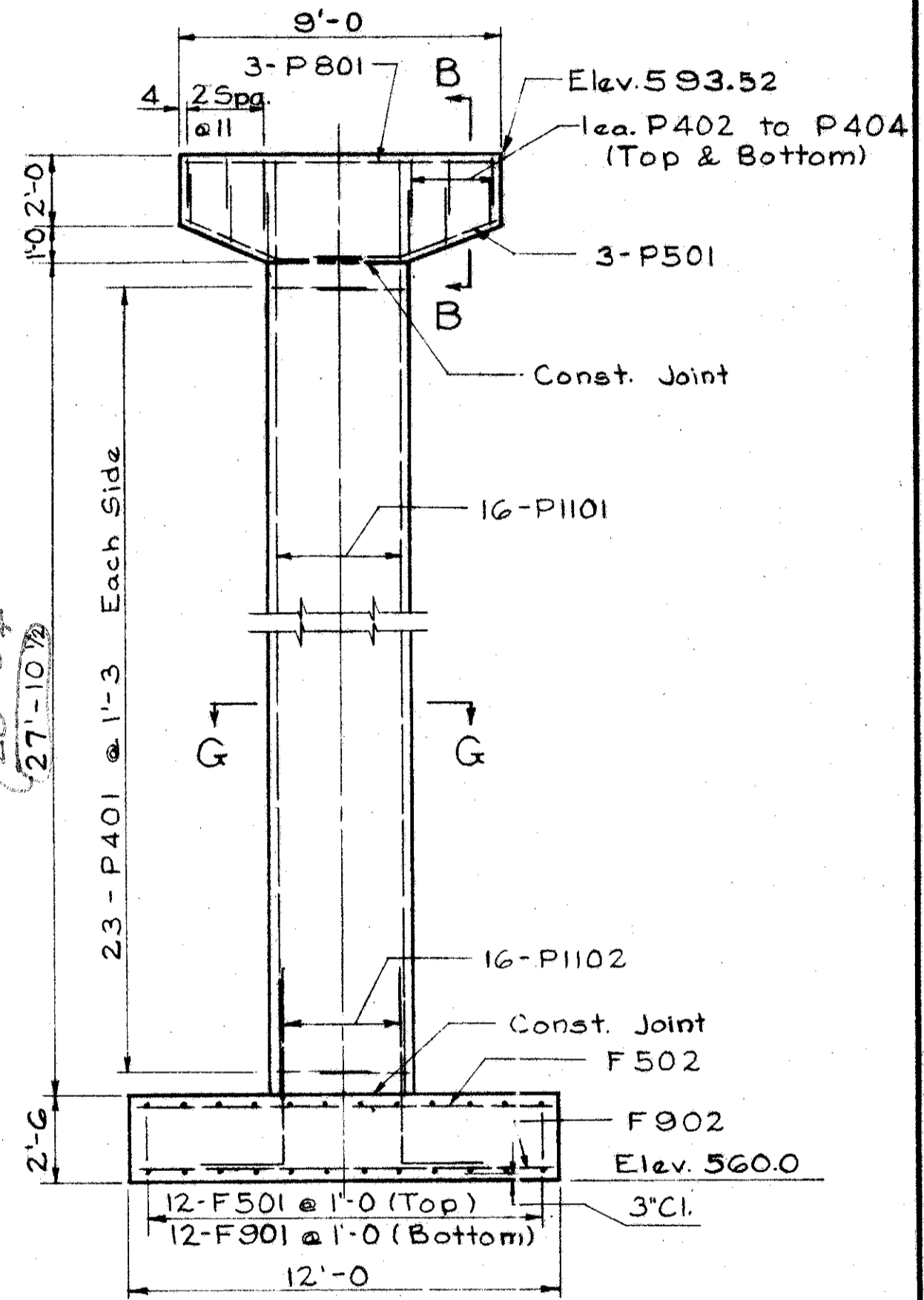
SECTION G-G



PLAN (Column C-10)



PLAN (Pier 6)



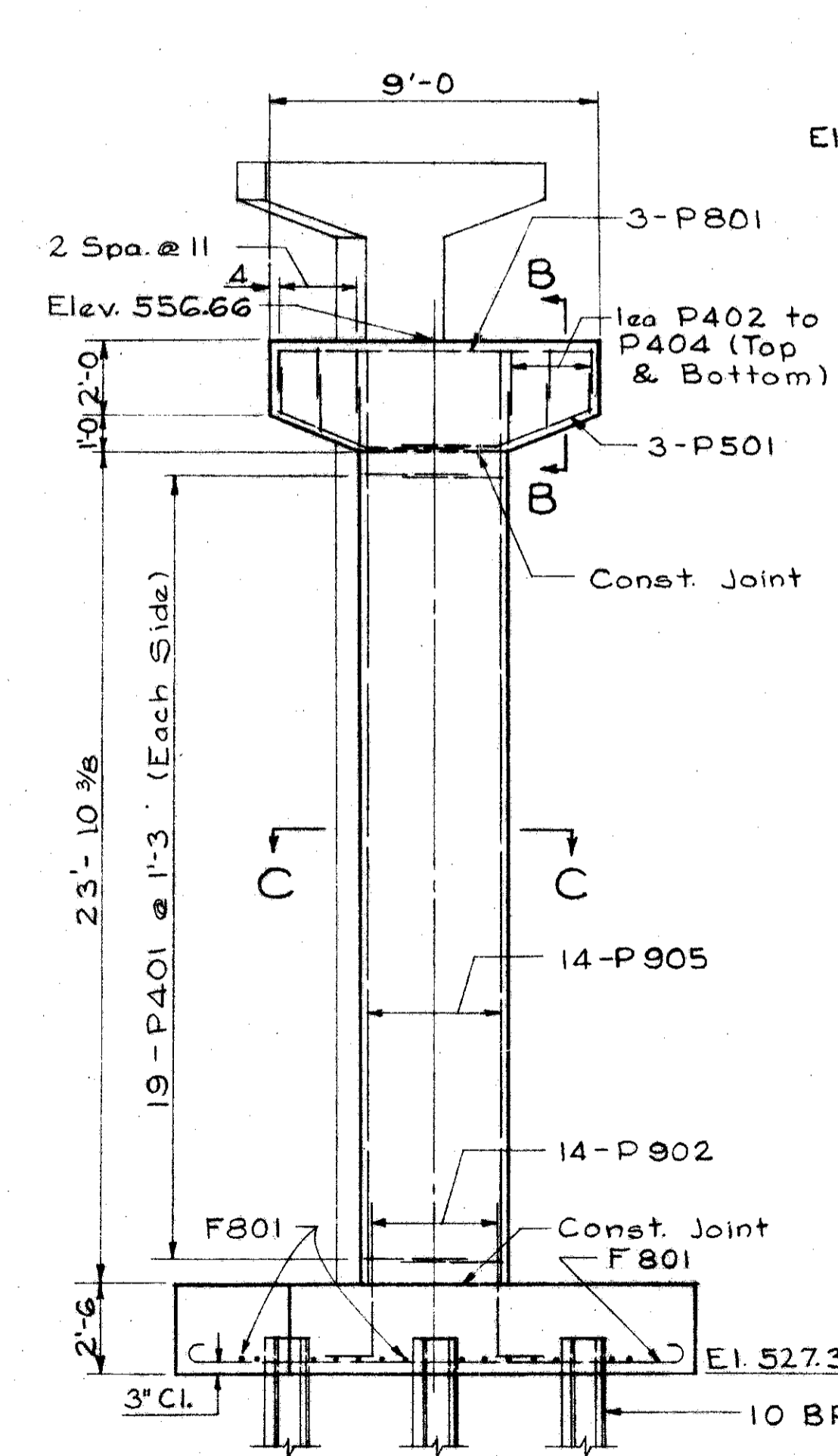
ELEVATION (Pier 6)

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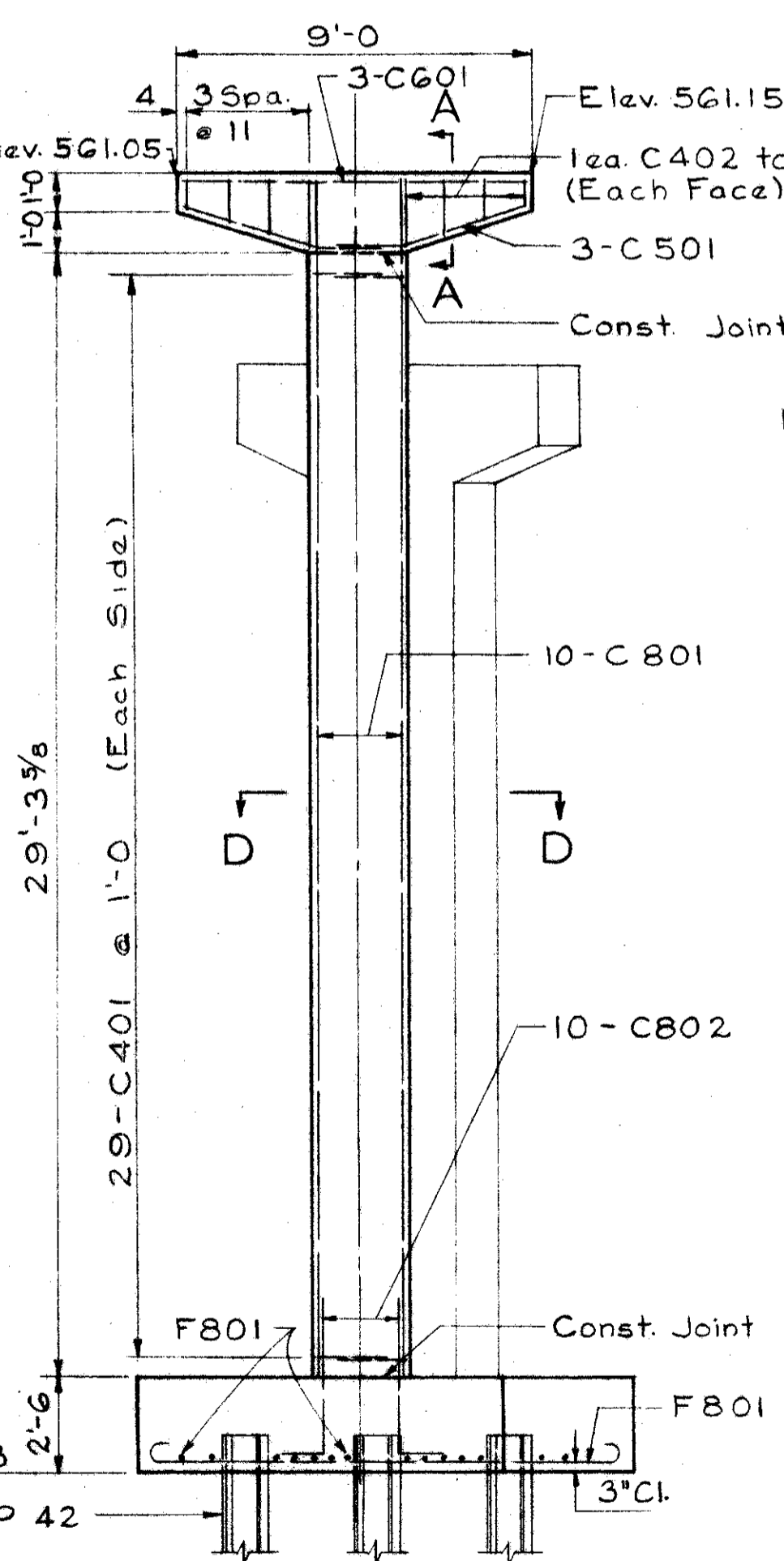
CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

PIER AND COLUMN DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.

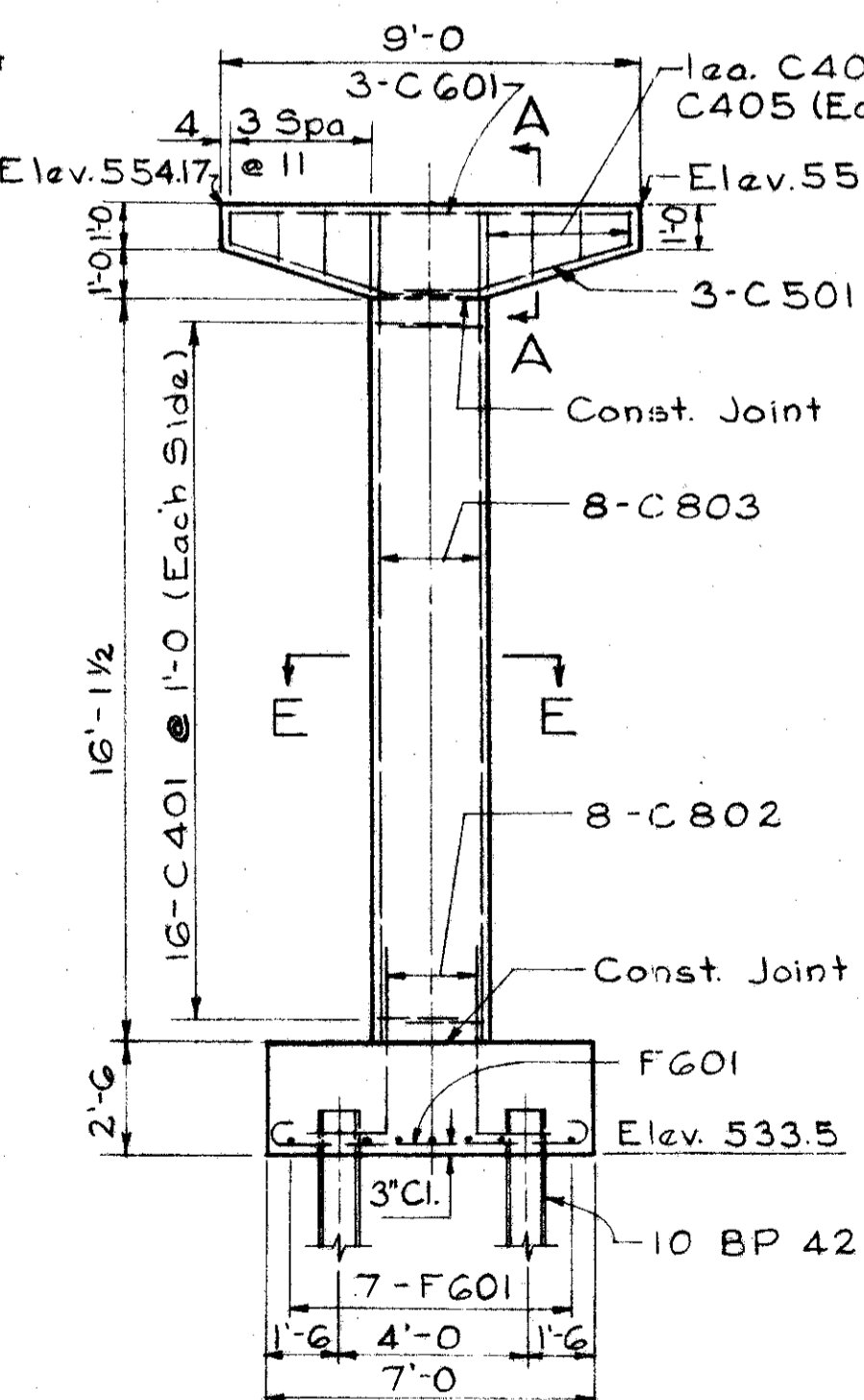
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
J.G.T.	J.G.T.		LEN.	W.W.	3-8-65	9-27-66



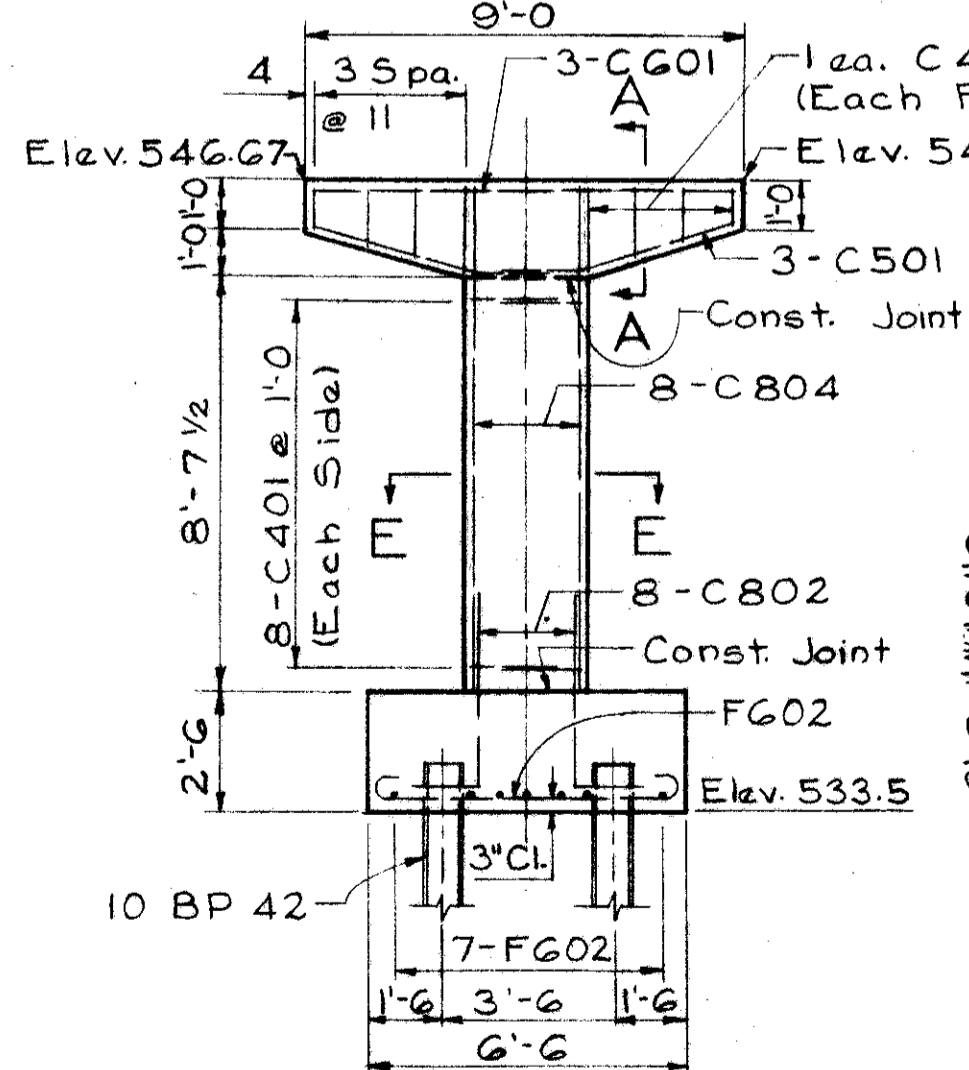
ELEVATION A-A (Pier 1)



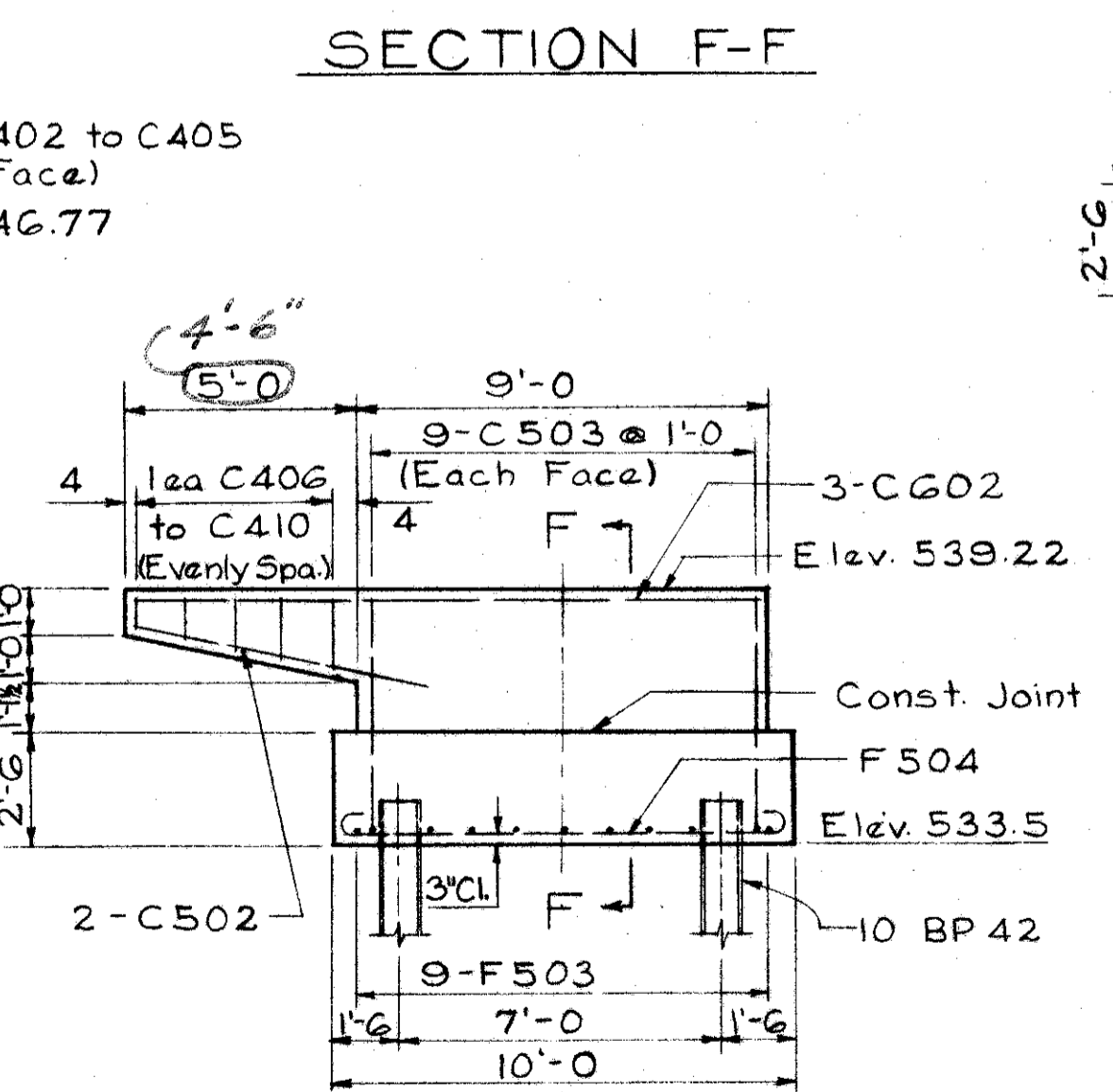
ELEVATION B-B (Column C-7)



ELEVATION (Column C-8)

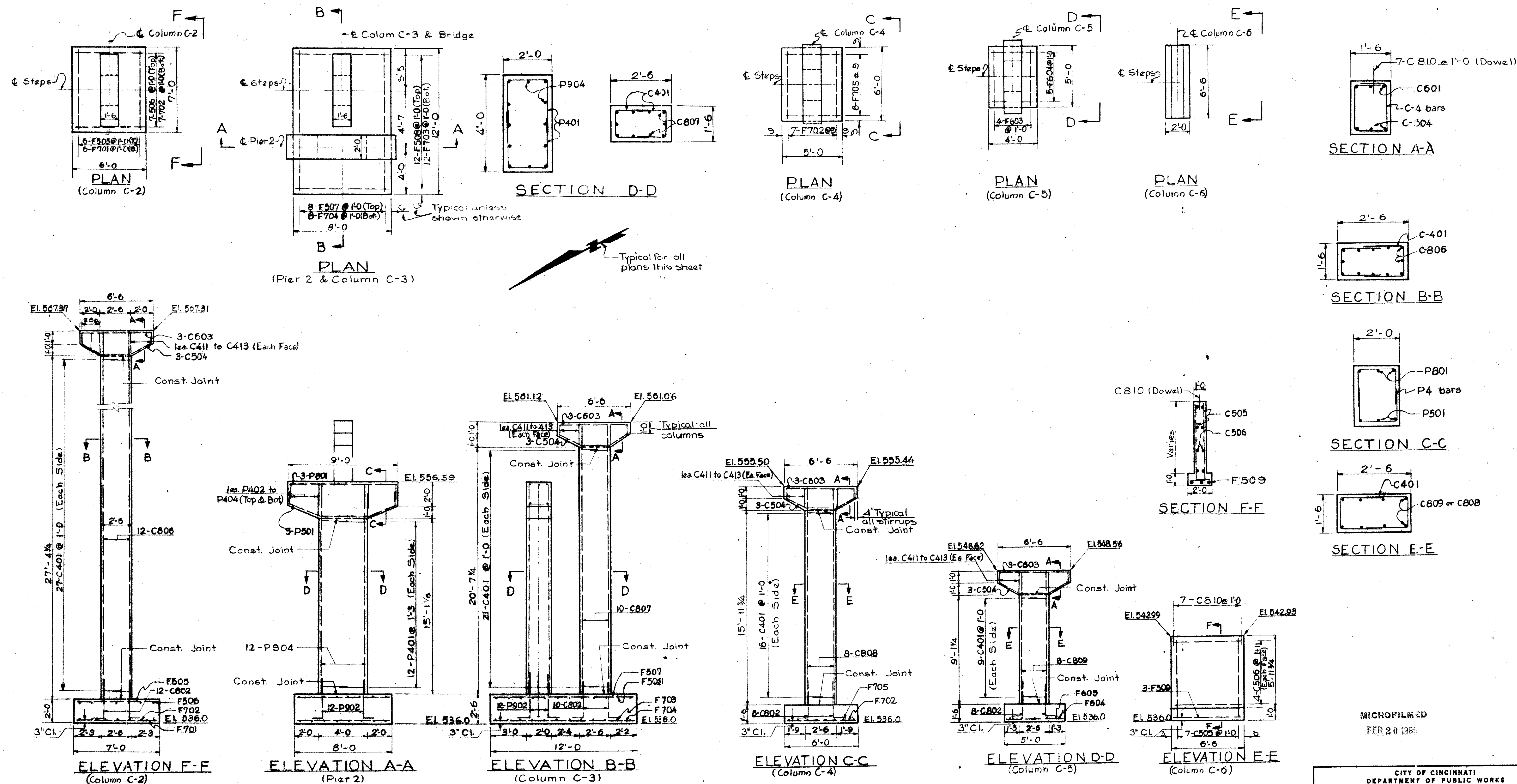


ELEVATION (Column C-9)



ELEVATION (Column C-10)

Bridge Seat Reinforcing: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.
Footings for Pier 6 shall extend a minimum of 3" into undisturbed rock or to the elevation shown, whichever is lower.
Foundation Bearing Pressure: Footing for Pier 6 is designed for a maximum bearing pressure of 3 tons per sq. ft.



For ground wire to be placed at pier 3 see note on sheet 213.

Bridge Seat Reinforcing: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.

Foundation Bearing Pressure: Footings are designed for a maximum bearing pressure of 3 tons per sq. ft.

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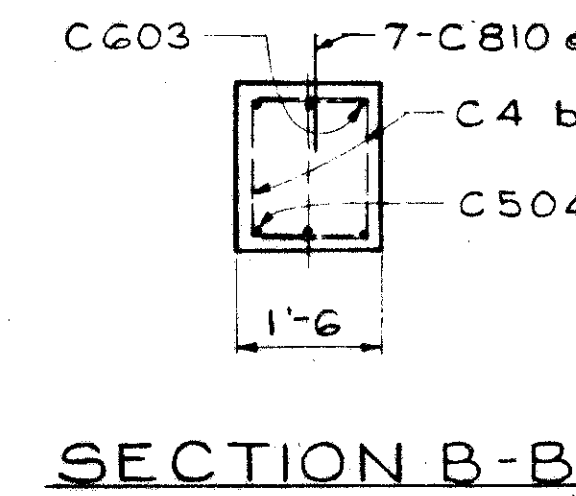
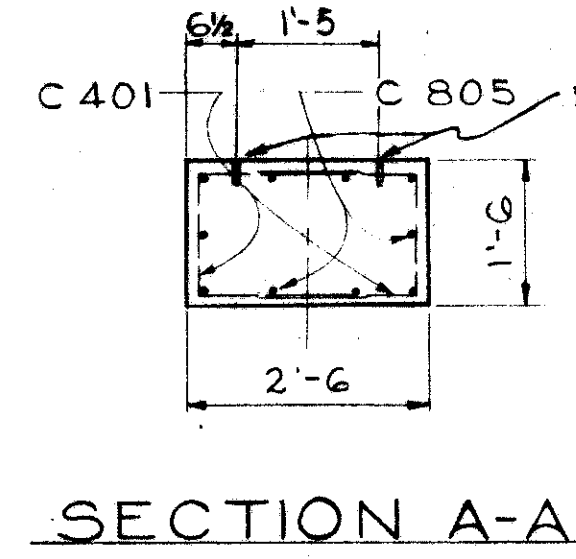
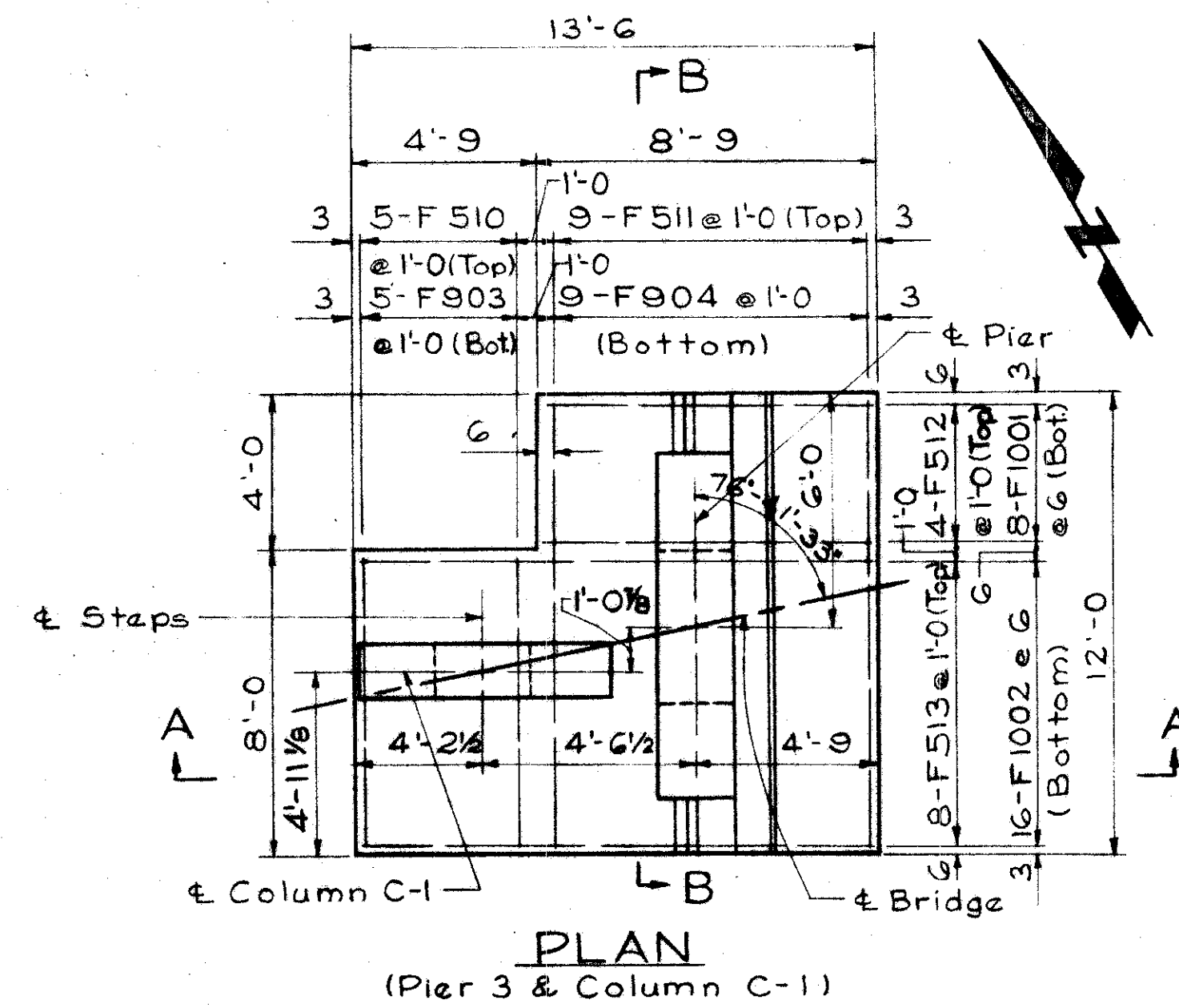
CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

PIER AND COLUMN DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.

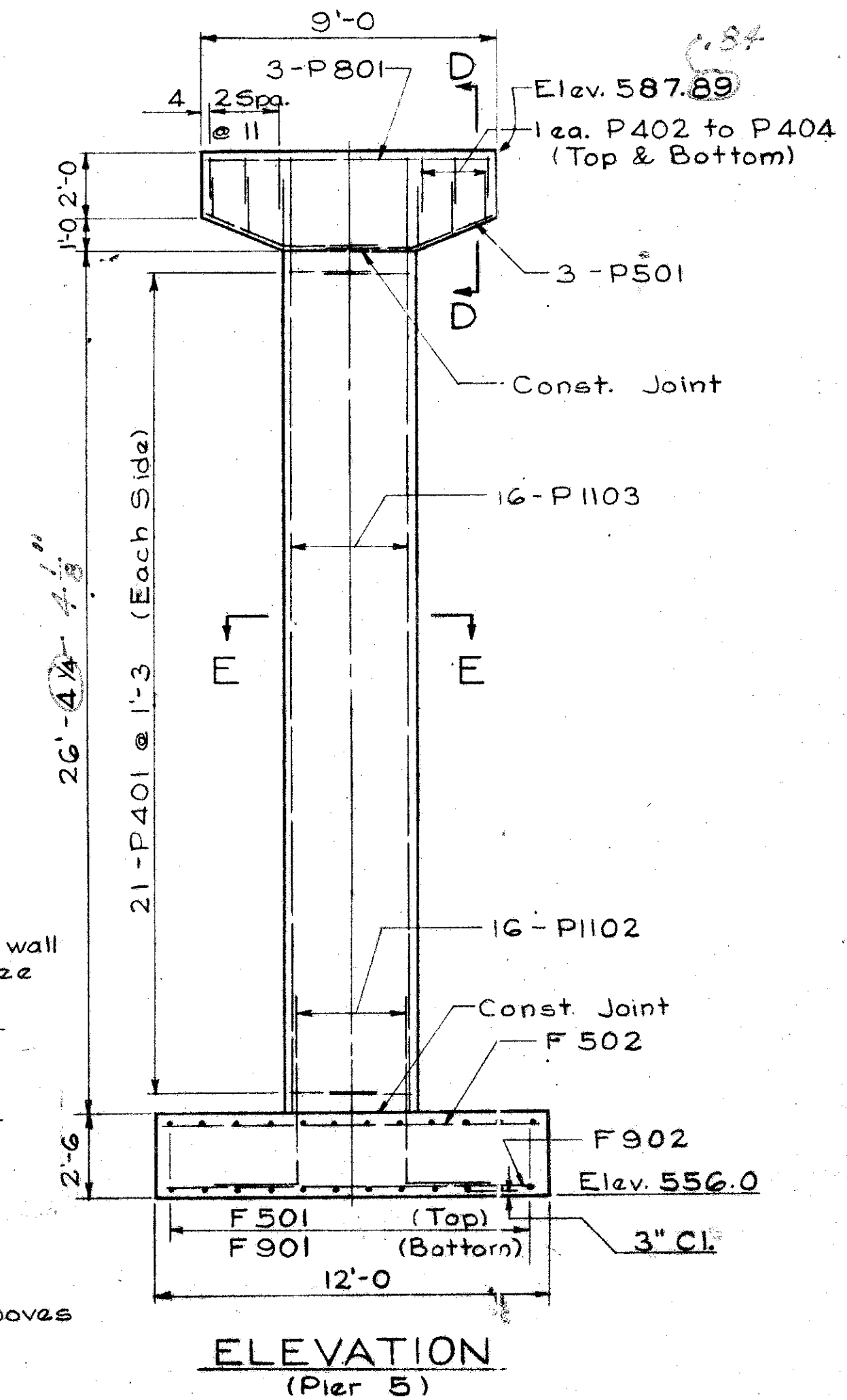
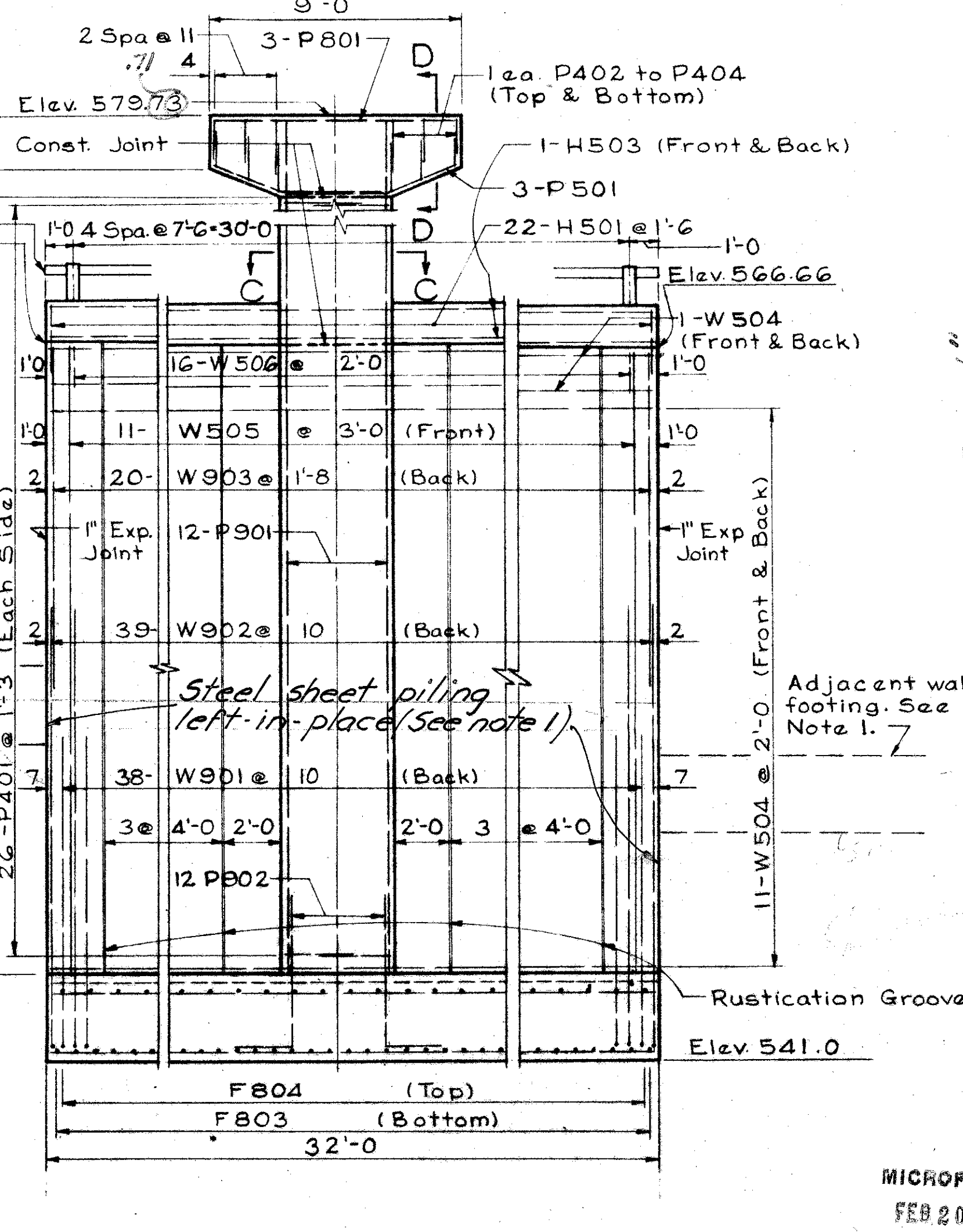
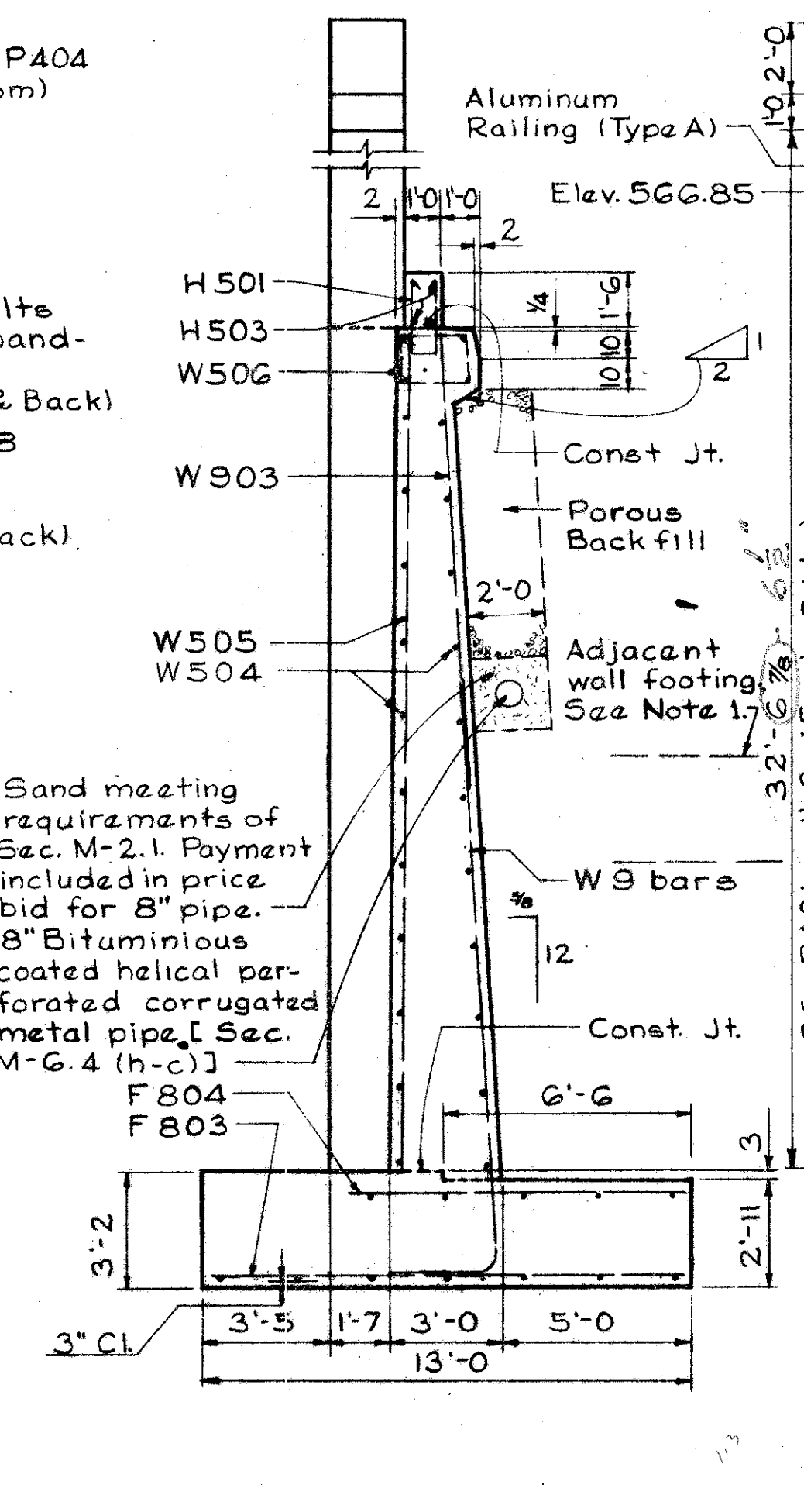
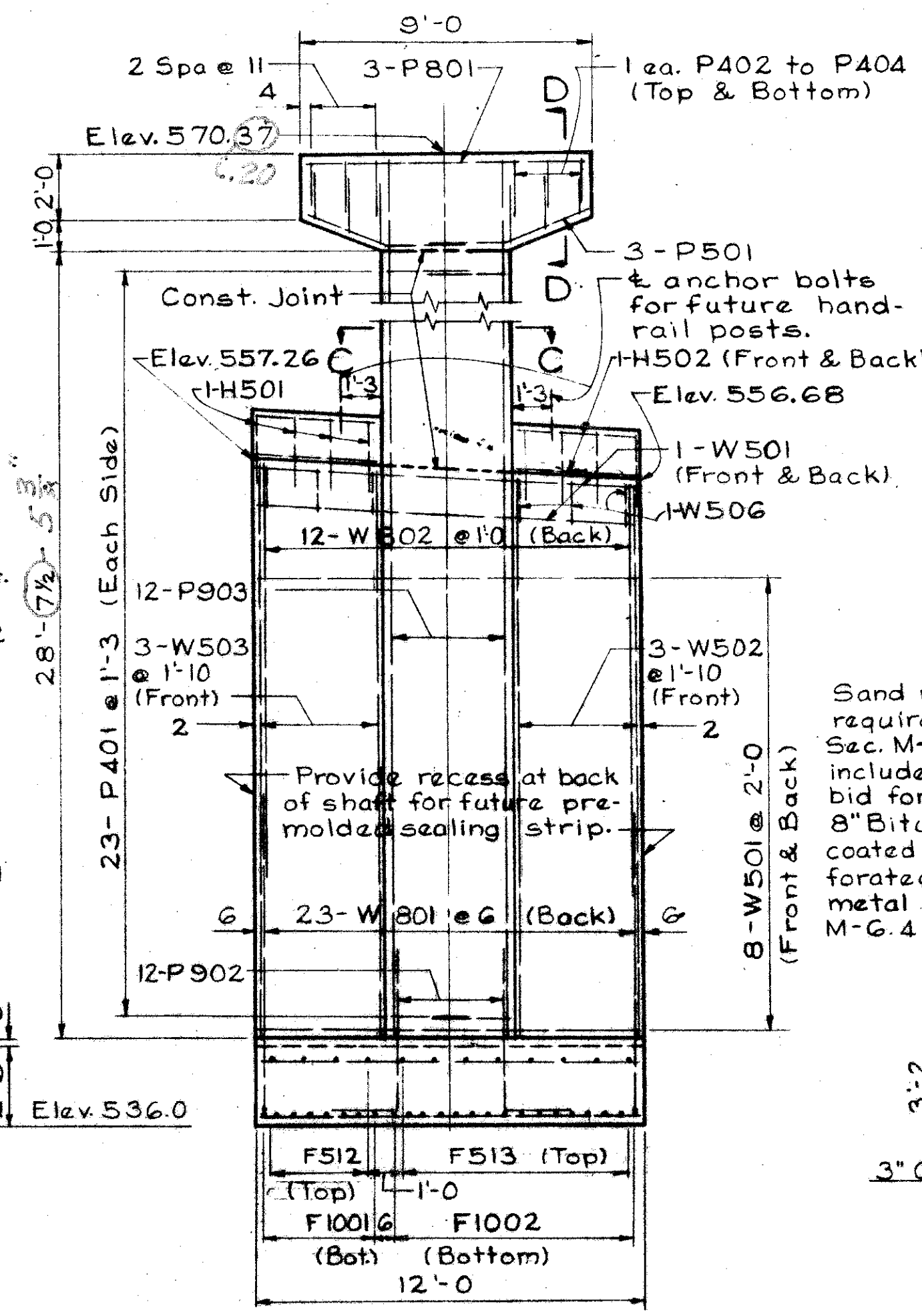
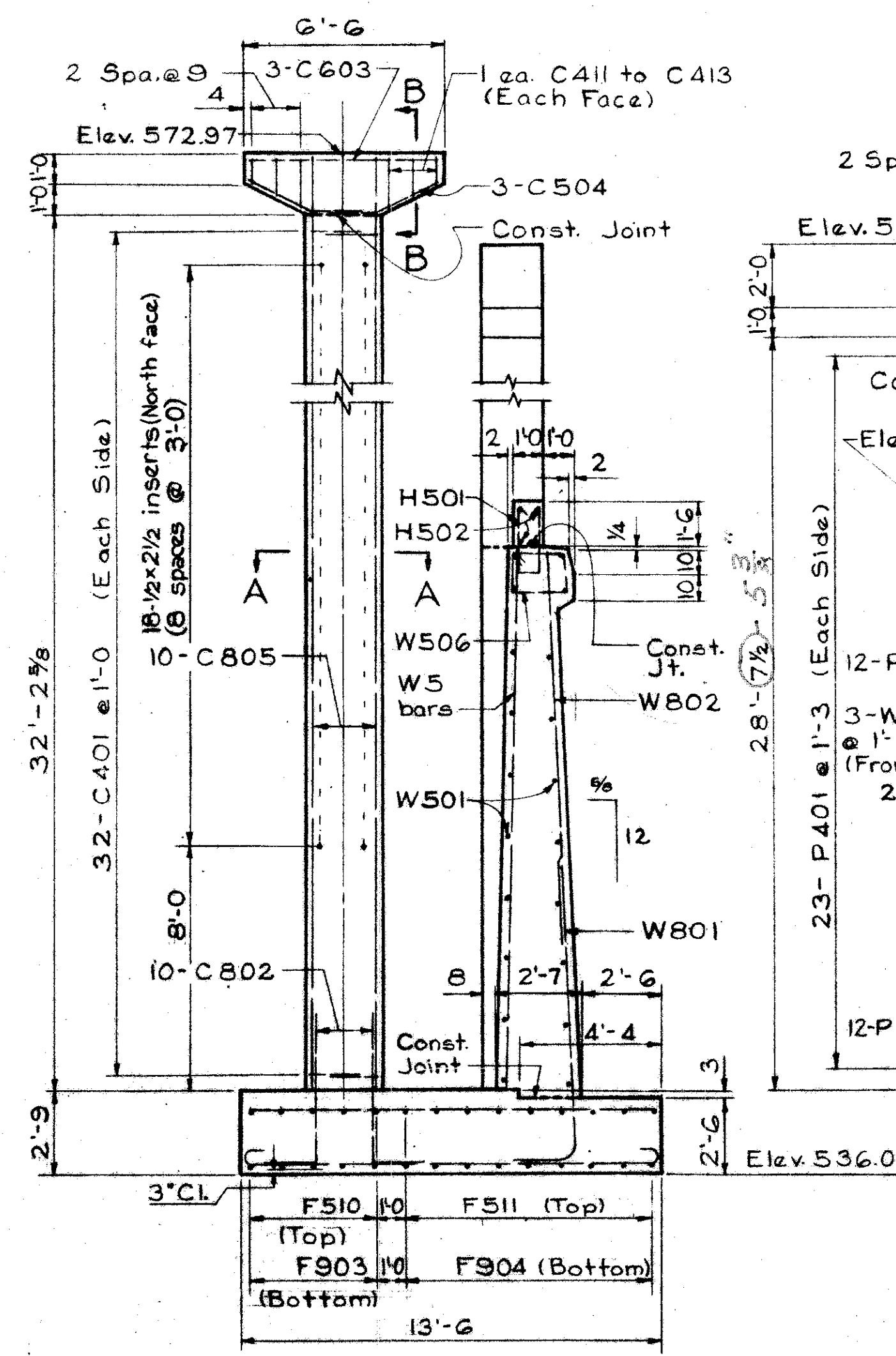
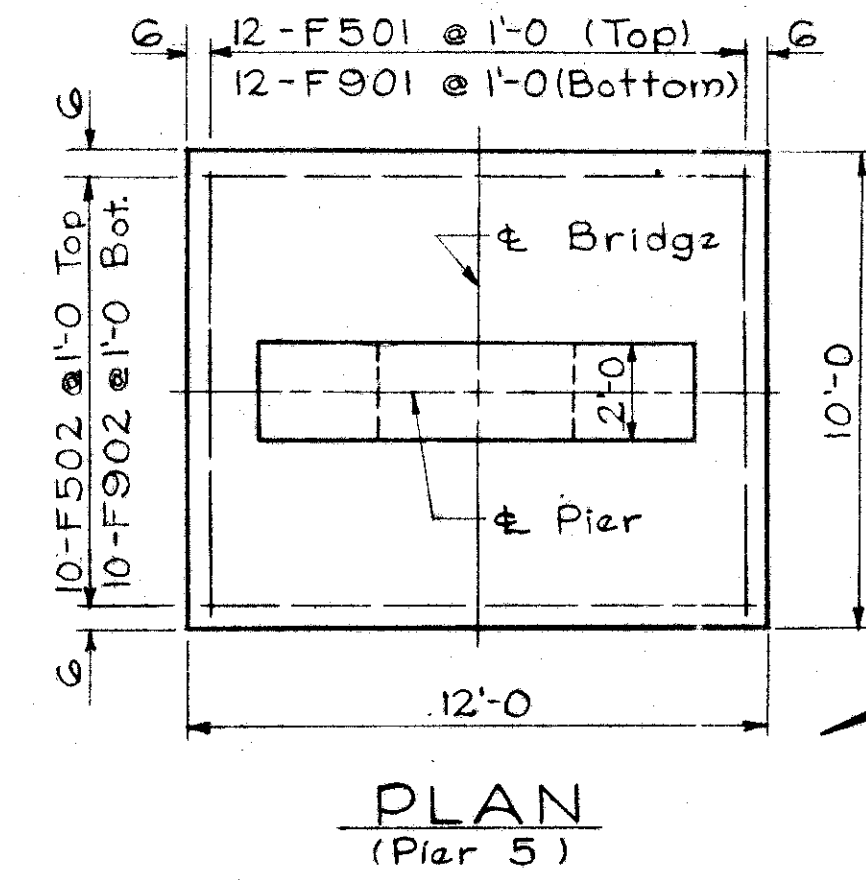
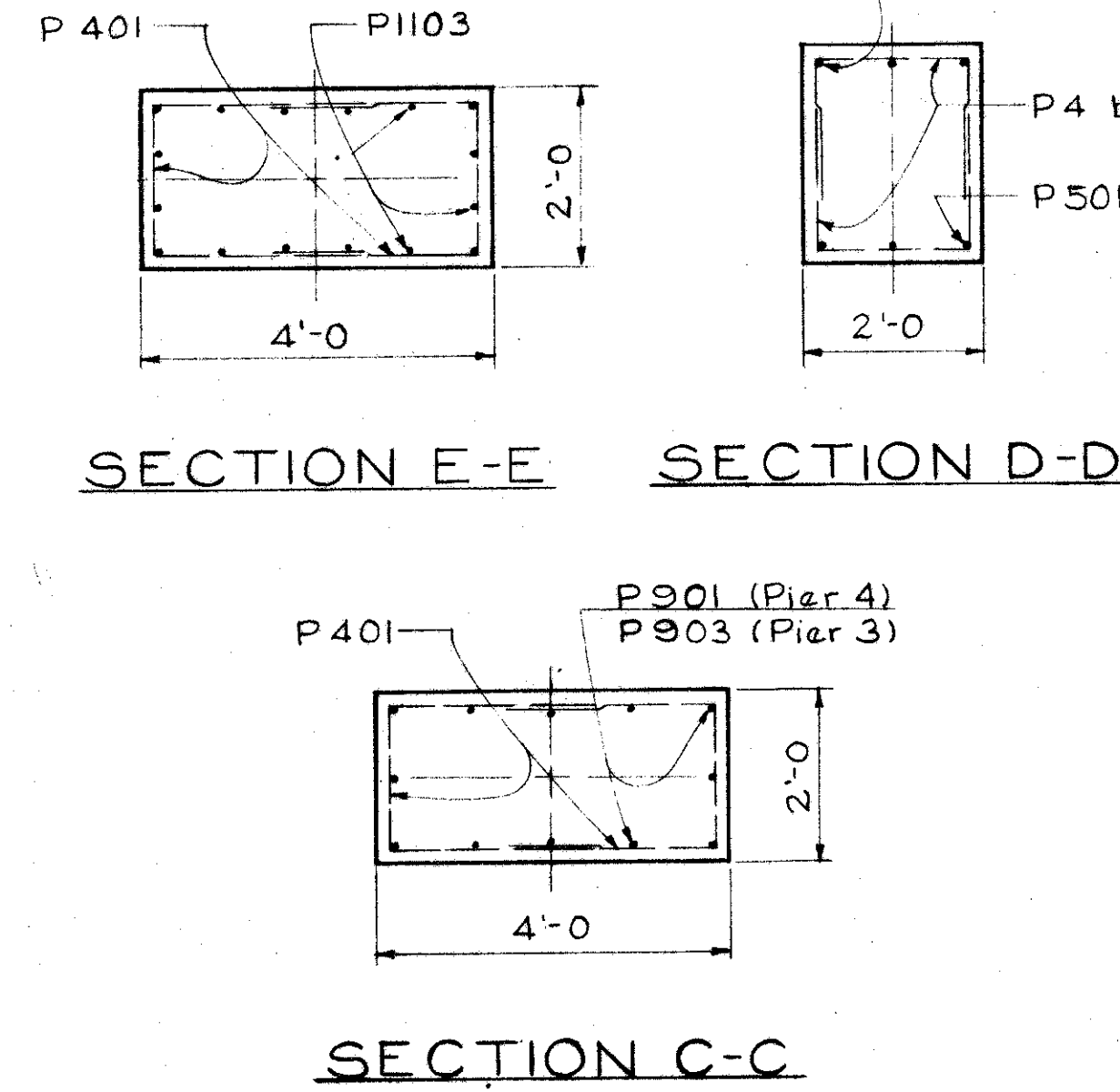
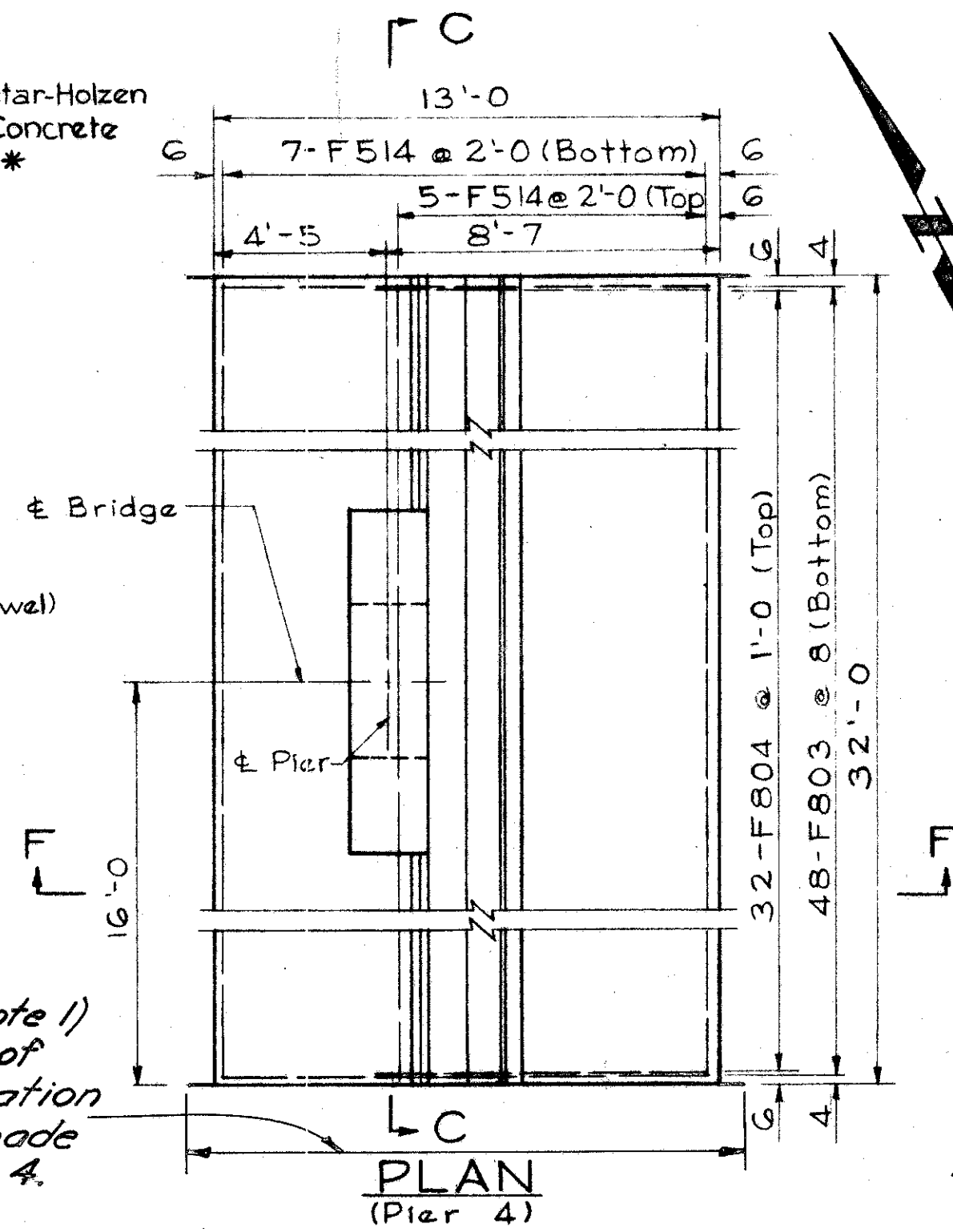
CINCY BRIDGE NO. 7

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
L.E.N.	L.E.N.		J.G.T.	W.W.	3-8-65	

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		



Steel sheet piling (see note 1) shall extend the full width of the excavation, at the elevation of adjacent wall footings, made for the construction of Pier 4.



Parapet and anchor bolts shall be included under Item S-14 Railing for payment.
* Inserts to be furnished by the C.&S.B.T. Co. Installation cost shall be included in price bid for Item S-1. Class "C" Concrete, piers above footings.

NOTE 1: If adjacent wall footing is not on bed rock, the following shall apply:
Steel Sheet Piling having a minimum section modulus of 3.4 in³ per foot of wall, shall be driven into the ground at the offset in the wall footings where indicated. This sheet piling shall be driven to bed rock and shall extend to the bottom of the higher footing. This sheet piling is to be braced adequately at the top and is to be left in place. Payment will be made at the price bid per square foot measured from the bottom of sheet piling as driven to the bottom of the higher footing; and on the sides to the actual limits as driven. Used piling in good condition may be used. Mill test reports are not required.
For details of expansion joints and rustication grooves, see Sheet Nos 245 & 246.

Bridge Seat Reinforcing: Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.
Footings for Piers 4 and 5 shall extend a minimum of 3" into undisturbed rock or to the elevation shown, whichever is lower.
Foundation Bearing Pressure: Footings are designed for a maximum bearing pressure of 3 tons per sq. ft.
For Typical Railing Details, see Standard Drawing AR-1-57.

For ground wire to be placed at Pier 5 see sheet 213.

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DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

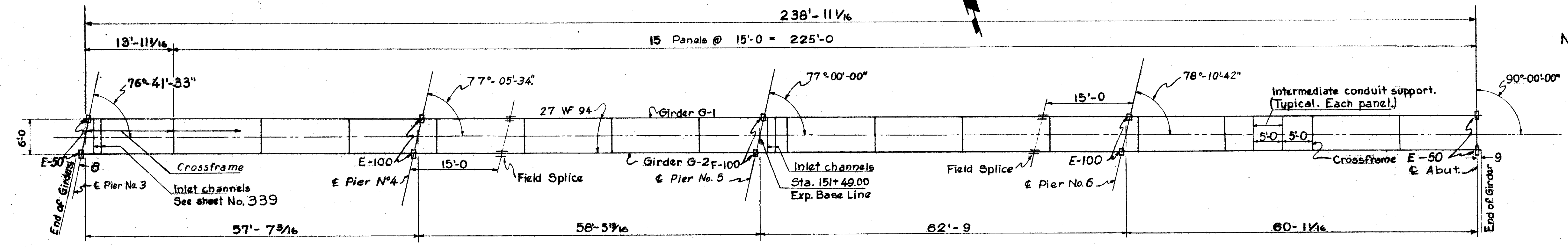
PIER AND COLUMN DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.

CINCY BRIDGE NO. 7

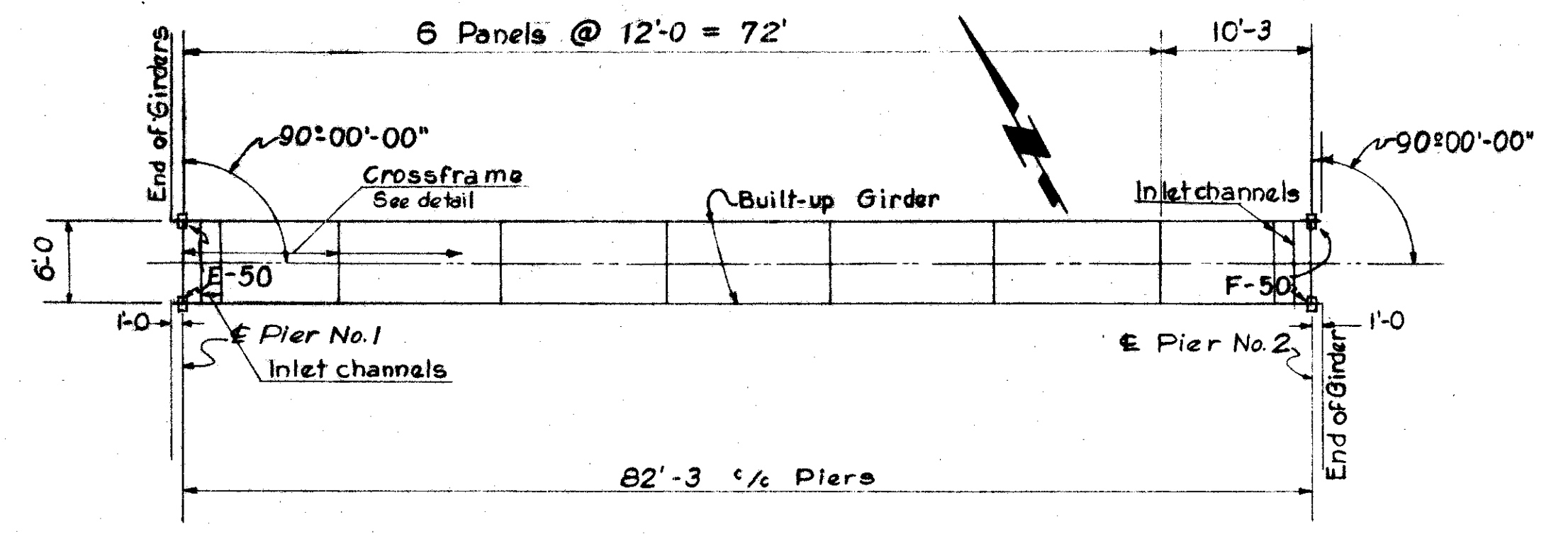
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J.G.T.	J.G.T.		L.E.N.	W.W.	3-8-65	J.P.

HAMILTON COUNTY
HAM-71-(15G)(2.51)

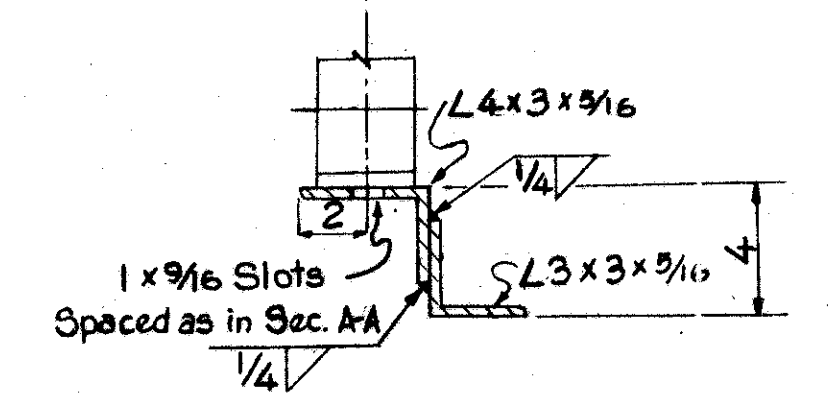
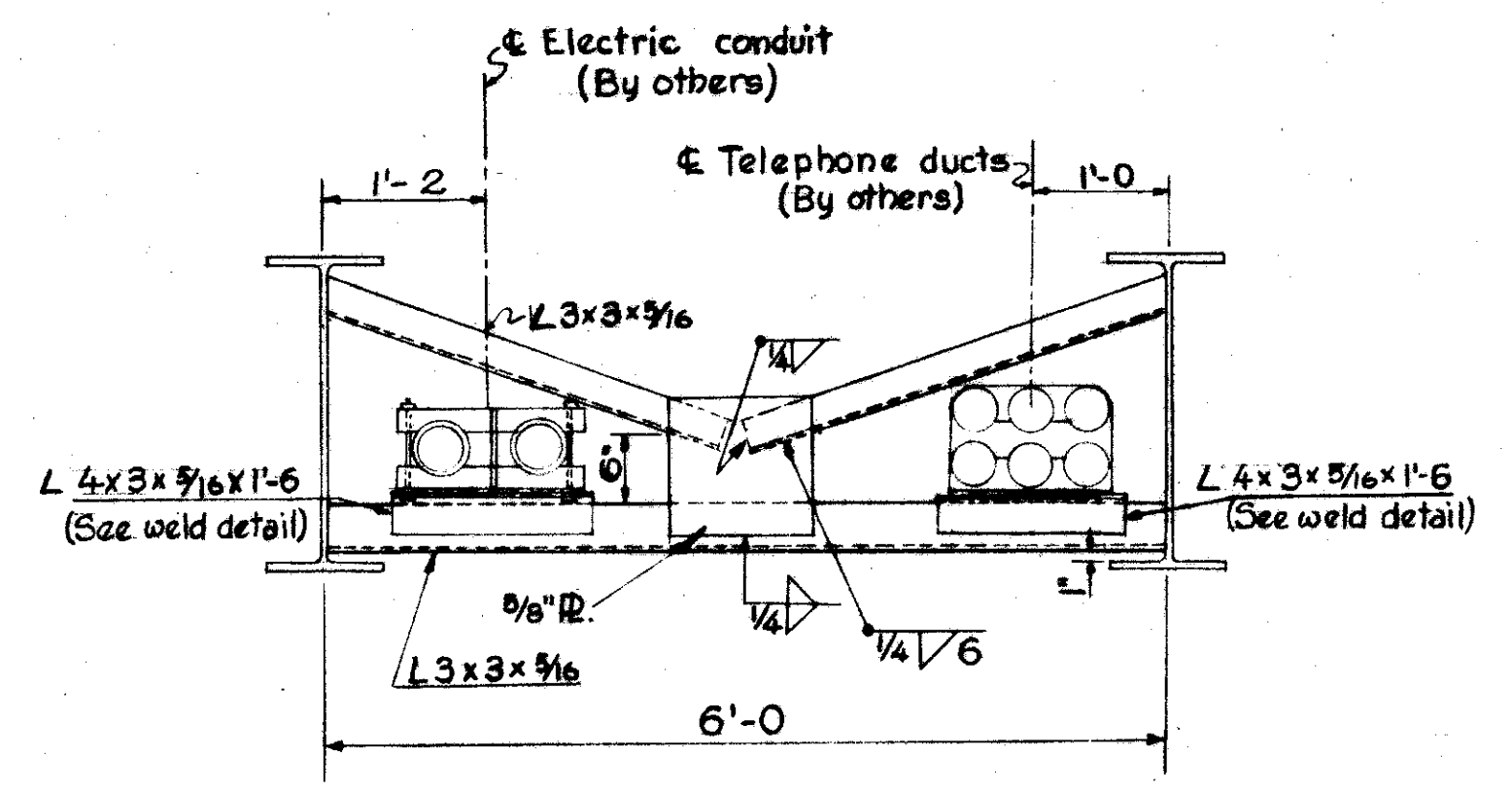
Note:
For Bolted Beam Splice Details, see Standard Drawing No. SD-2-64.
For Fixed and Sliding Bearings: Details see Standard Drawing No. FSB-1-62. Increase top plate thickness as necessary. End of girders and end crossframes shall be vertical. Cut flanges parallel to end of deck.
Angles 4x4x1/8 & 4x3x5/16 for conduit supports shall be chargeable to "Cincinnati Gas & Electric Co." and to "C.S.B. Telephone Co."



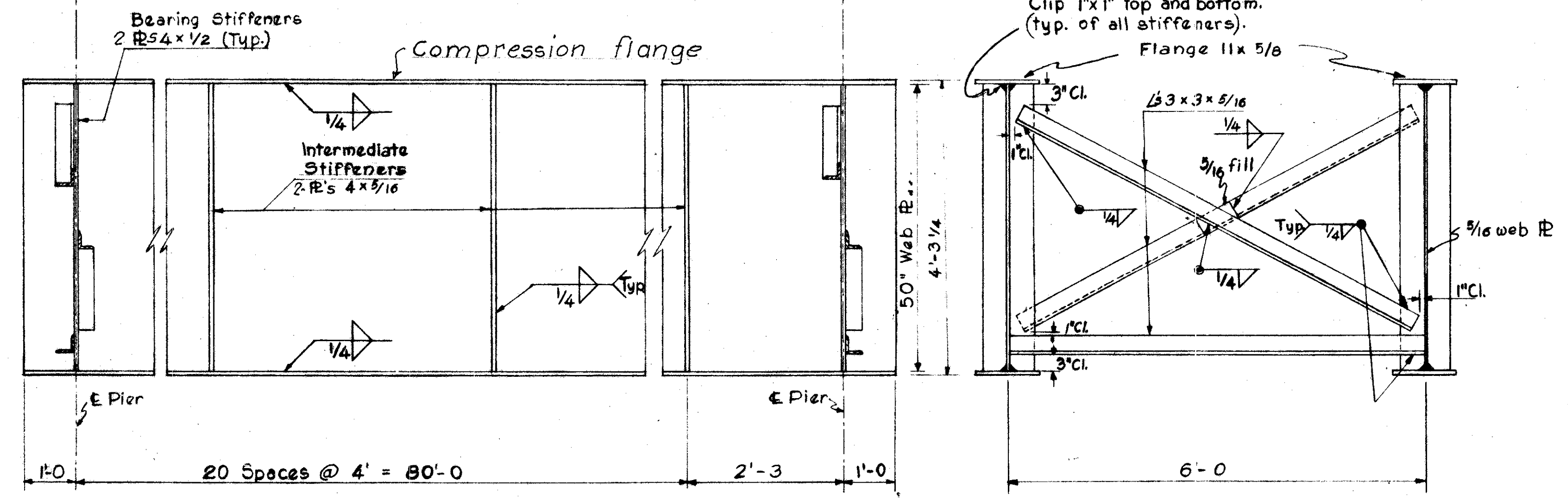
FRAMING PLAN ON EXPRESSWAY - FOUR SPANS



FRAMING PLAN ON GILBERT AVE.

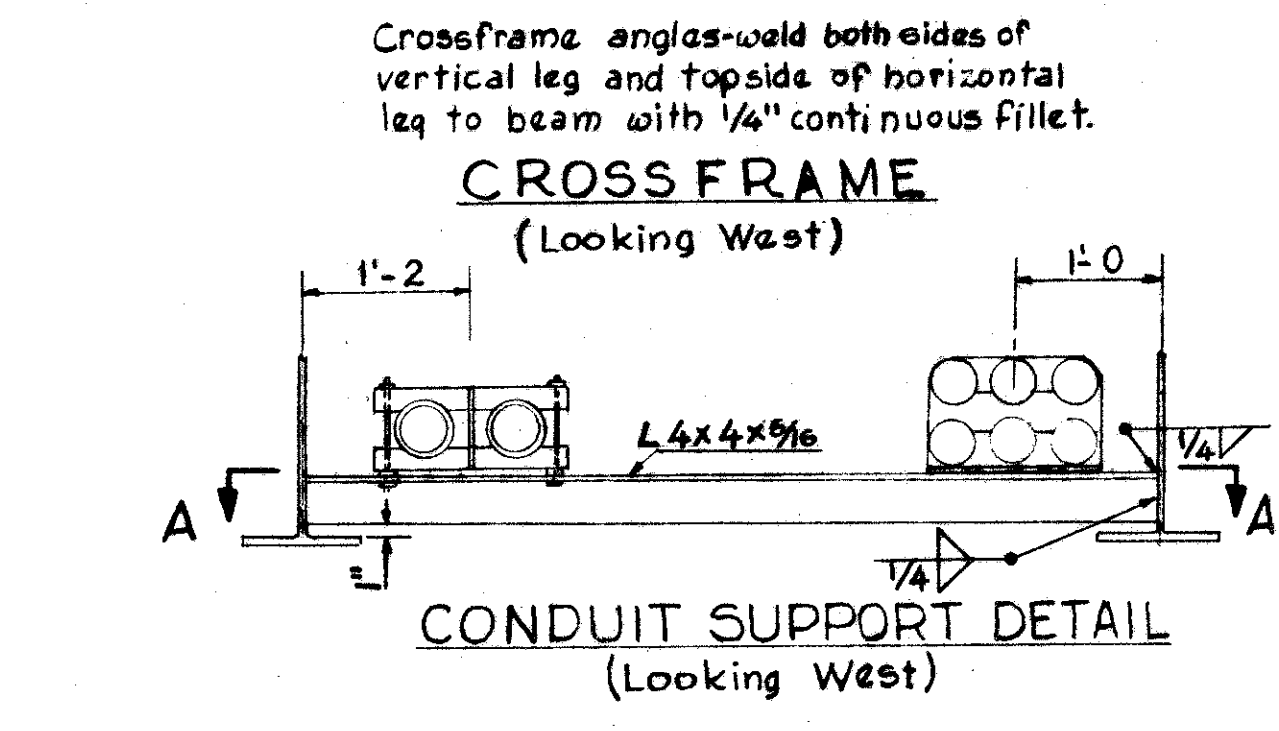


WELD DETAIL



GIRDER ELEVATION - (INSIDE)

CROSSFRAME



CONDUIT SUPPORT DETAIL (Looking West)

DEFLECTION & CAMBER OF BEAMS		PIER No. 3								PIER No. 4								PIER No. 5								PIER No. 6								Abut.	
LOCATION	LOCATION	E		1/4		3/4		1/4		E		3/4		1/4		E		3/4		1/4		E		3/4		Abut.									
		Length % bearings	57.58'	58.48'	62.69'	59.46'	57.62'	58.48'	62.81'	60.72'	57.62'	58.48'	62.81'	60.72'	57.62'	58.48'	62.81'	60.72'	57.62'	58.48'	62.81'	60.72'	57.62'	58.48'	62.81'	60.72'	57.62'	58.48'	62.81'	60.72'					
Deflection due to weight of steel.	1/8"	0"	1/32"	0"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"	1/32"	1/16"							
Deflection due to remaining dead load.	7/16"	1/32"	3/32"	1/16"	5/32"	1/4"	3/32"	1/4"	1/2"	1/32"	3/32"	1/4"	1/2"	1/32"	3/32"	1/4"	1/2"	1/32"	3/32"	1/4"	1/2"	1/32"	3/32"	1/4"	1/2"	1/32"	3/32"	1/4"							
Convexity required for vertical curve.	0	3 3/8"	4 7/8"	3 1/2"	3 1/8"	5 1/16"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"							
Sum of Deflection and Convexity.	9/16"	3 7/16"	4 9/16"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"							
REQUIRED CAMBER	none	3 7/16"	4 9/16"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"	4 1/8"	5 3/8"	3 3/8"	3 3/8"							

DEFLECTION & CAMBER OF GIRDER			
LOCATION	WELDED BUILT-UP GIRDER		
	1/4 Span	1/2 Span	3/4 Span
Deflection due to weight of steel.	1/4"	5/16"	1/4"
Deflection due to remaining dead load.	3/4"	1 1/16"	3/4"
Convexity required for vertical curve.	2 1/2"	5"	2 1/2"
Sum of deflection and convexity.	3 1/2"	6 3/8"	3 1/2"
REQUIRED CAMBER	3 1/2"	6 3/8"	3 1/2"

NOTE:
Intermediate stiffeners of built-up girders shall have contact bearing with the compression flange, but may have a clearance of not more than 1/8 inch from the tension flange. In shop painting, care shall be taken to make certain that paint is forced through from one side to the other of the 1/8 inch opening.
Bearing stiffeners shall be grooved and fully butt-welded to the lower flange and fitted in close contact without welding to the upper flange.

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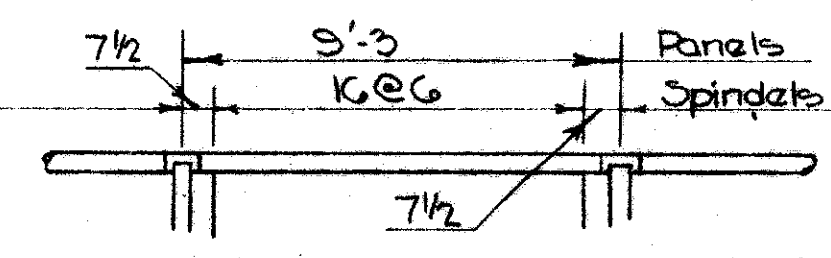
CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

STRUCTURAL STEEL DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.

CINCY BRIDGE NO. 7

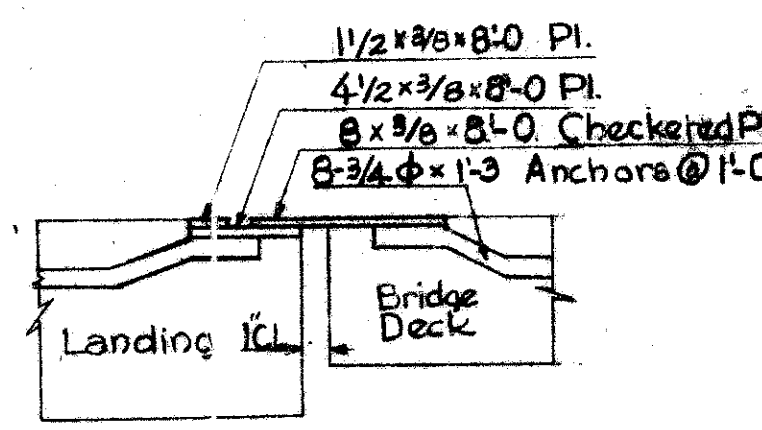
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LEN	LEN		RMB	W.W.	3-8-65	

HAMILTON COUNTY
HAM-71-(1.5G)(2.51)

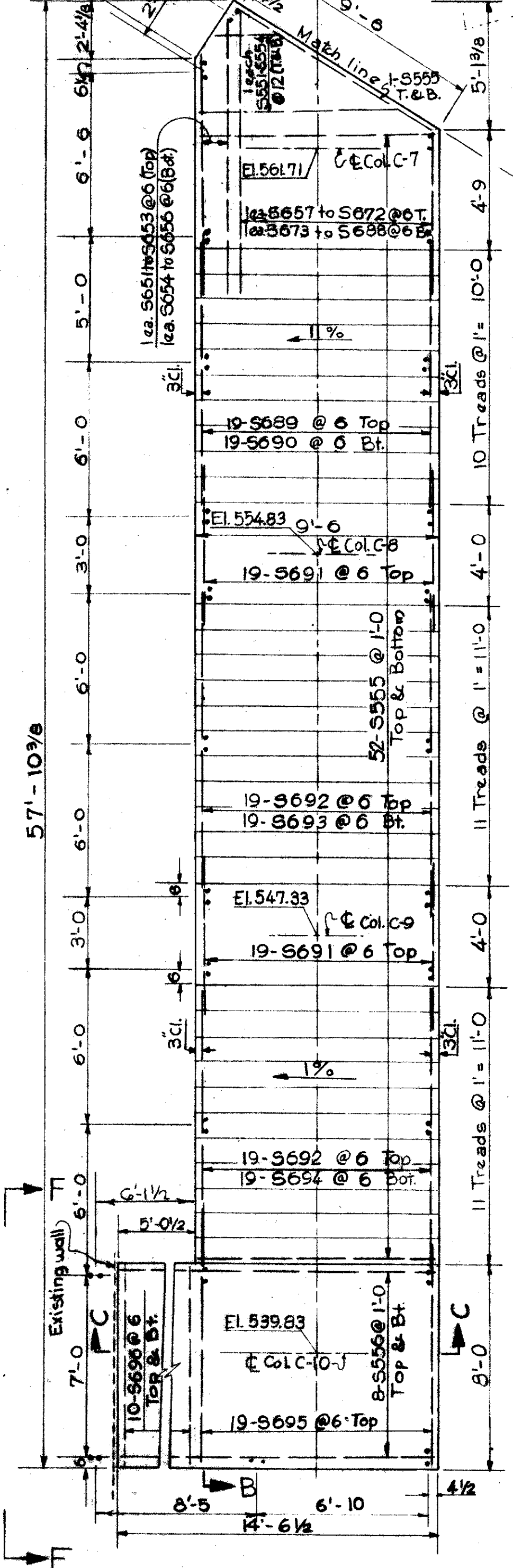
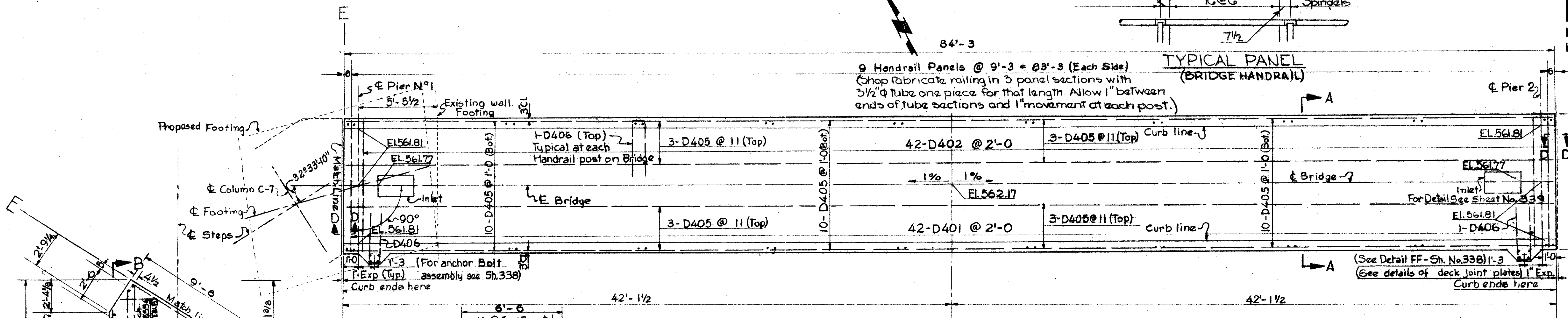


84'-3
9 Handrail Panels @ 9'-3 = 83'-3 (Each Side)
Shop fabricate railing in 3 panel sections with 3/2" tube one piece for that length. Allow 1" between ends of tube sections and 1" movement at each post.

TYPICAL PANEL
(BRIDGE HANDRAIL)

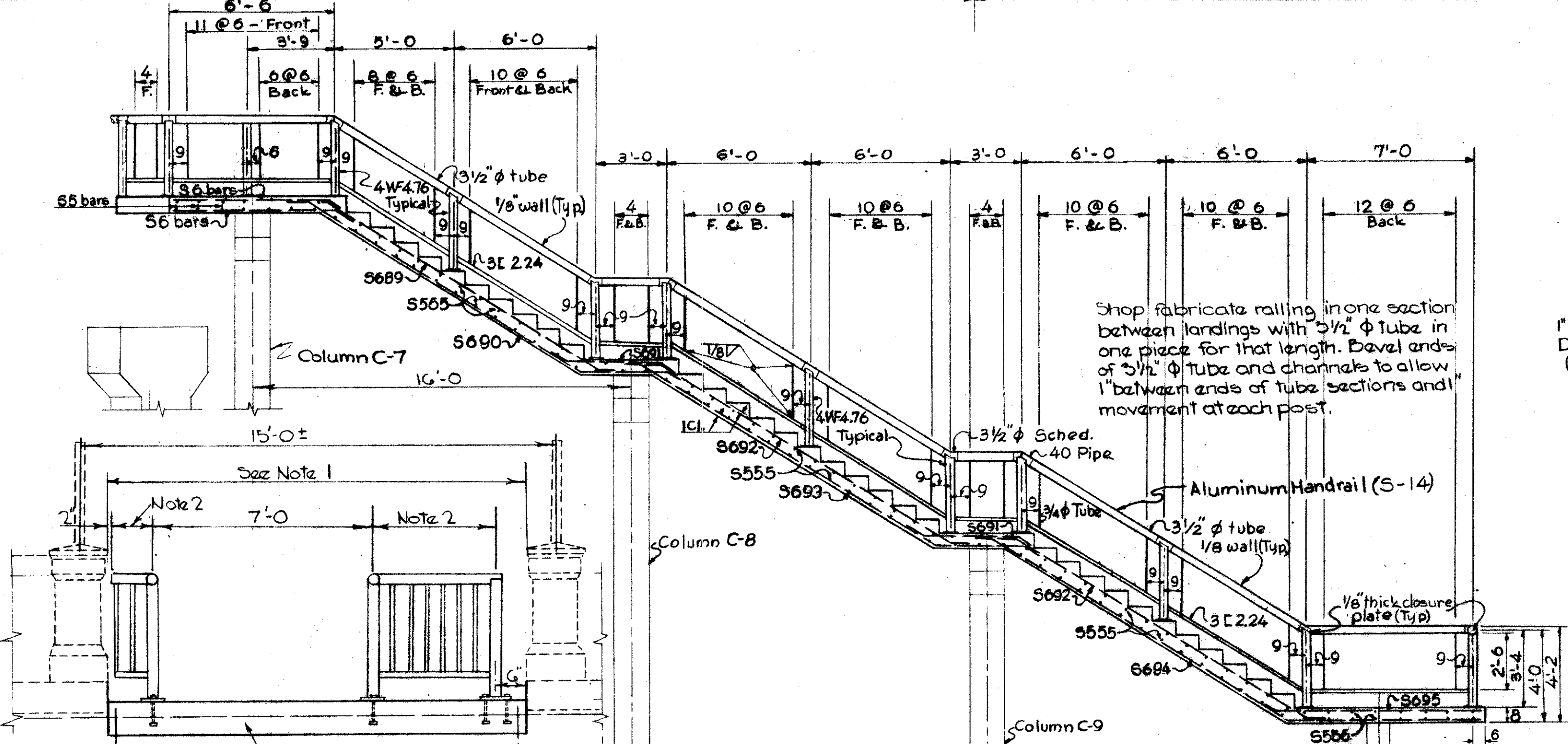


SECTION D-D
For details not shown see Joint Detail sheet.

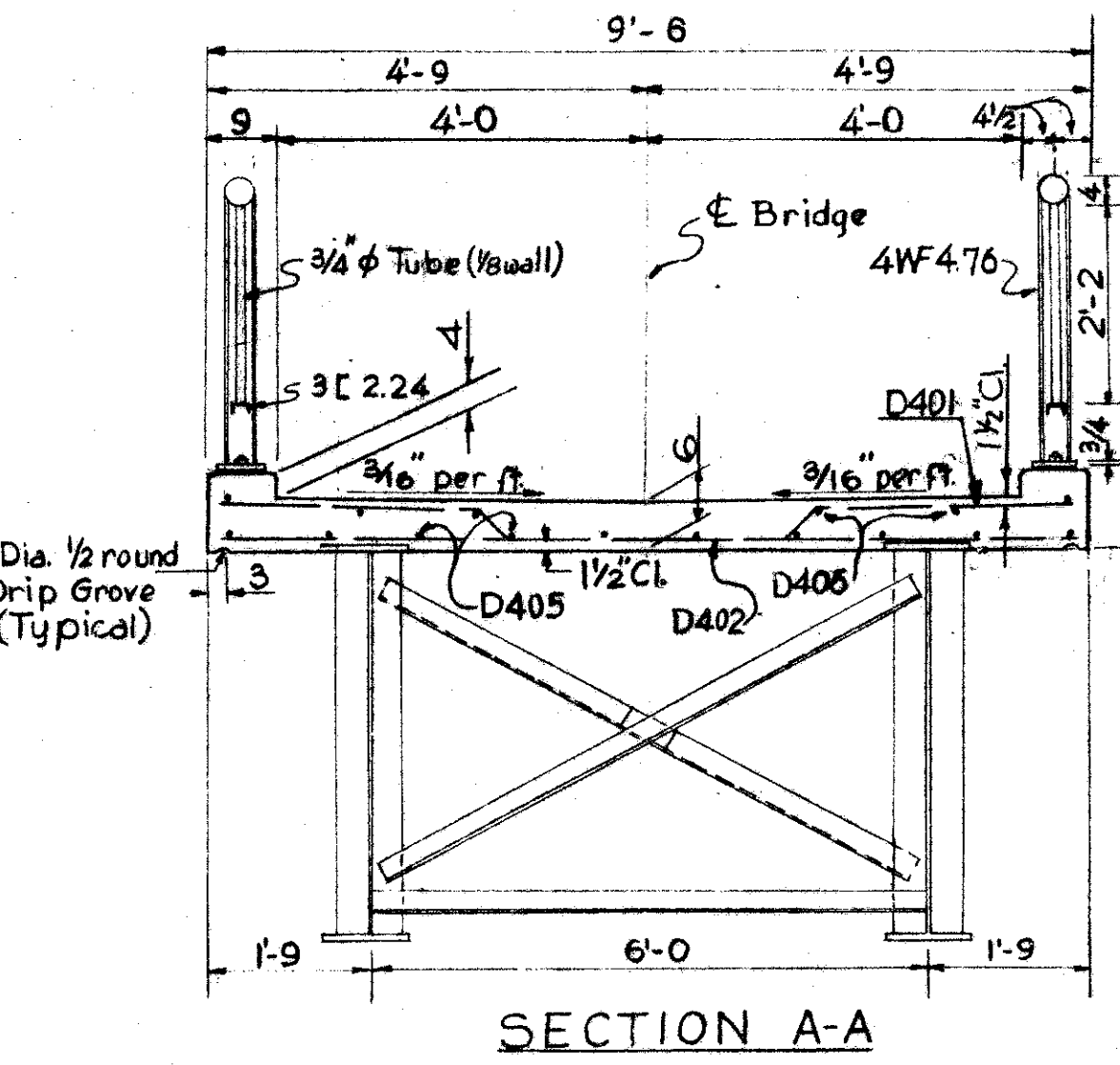


Note 1
The existing railing, wall coping and fence between posts shall be removed completely. Provide fence terminal at posts. Cut existing horizontal reinf. bars 1" behind cut concrete surfaces and refinish as directed by Engineer.
Note 2: Posts on wall coping shall be lined up with step railing as shown. Aluminum railing panel dimensions shall then be determined for fabrication with a nominal spindle c-c spacing not to exceed 6".

Wall & Coping Items:
Removal, S-22
Replacement, S-1 (Superstructure and Steps).
Fencing Items:
Removal of existing fencing between posts (Note 1) and providing terminal connections shall be included in cost bid for item I-26



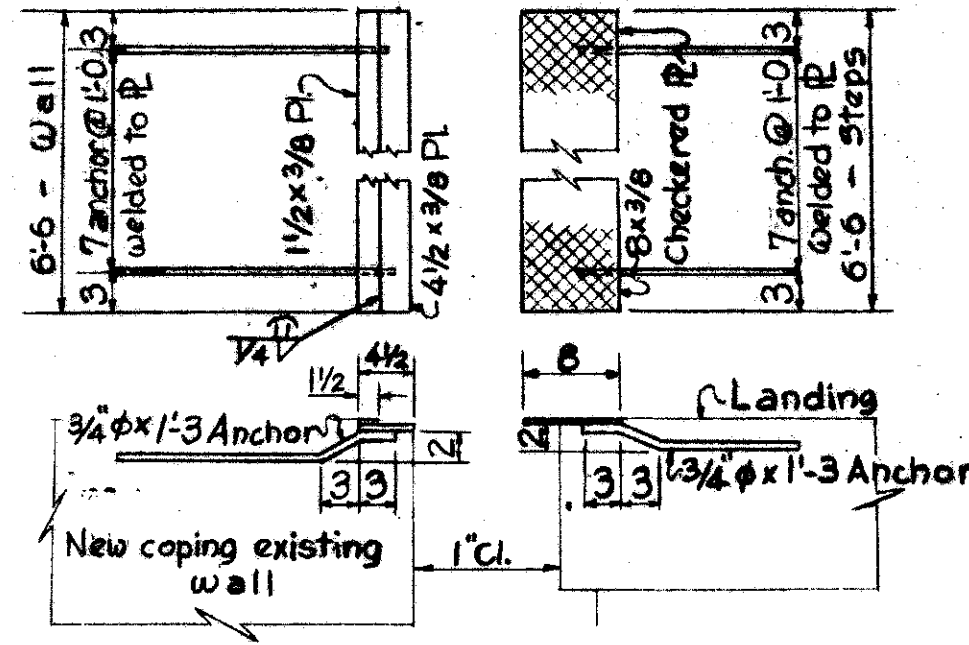
SECTION B-B



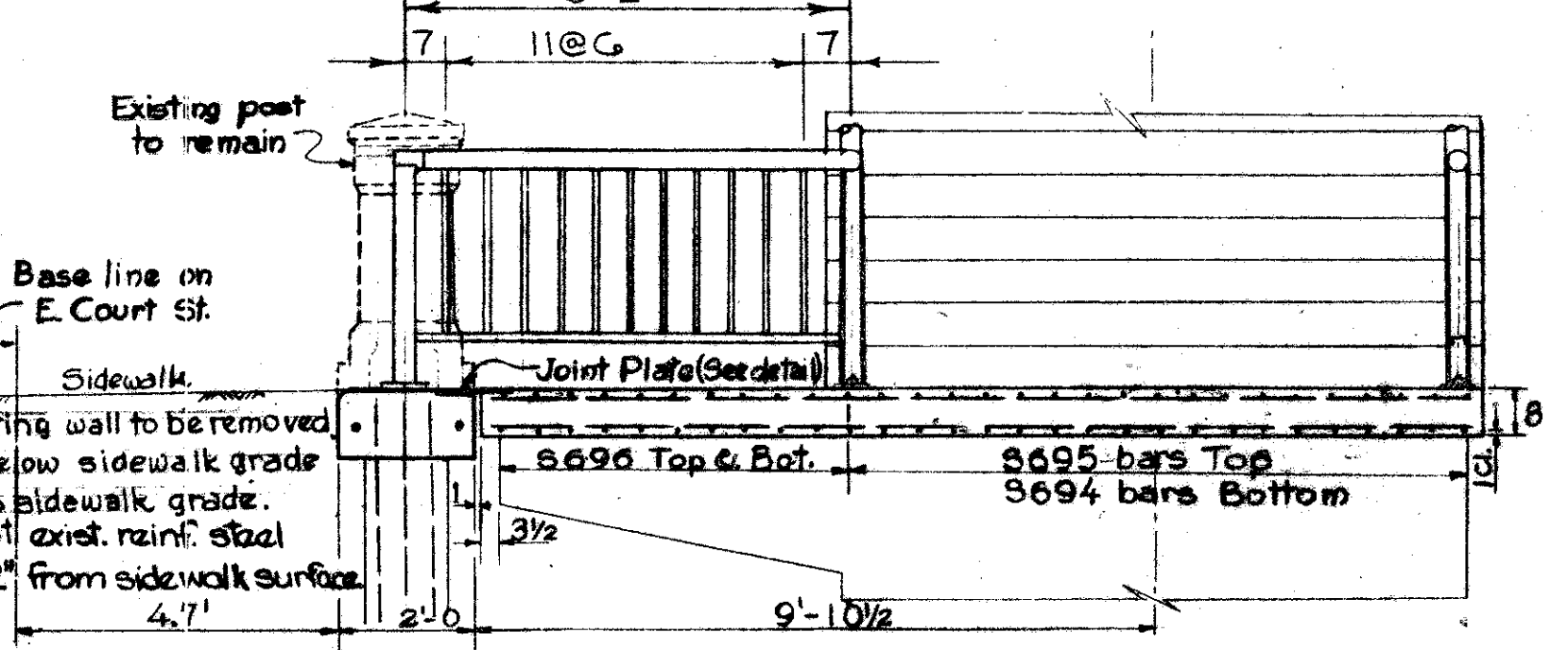
SECTION A-A
For lighting Details, see sheet No. 2/3.

TYPICAL HANDRAIL DETAILS

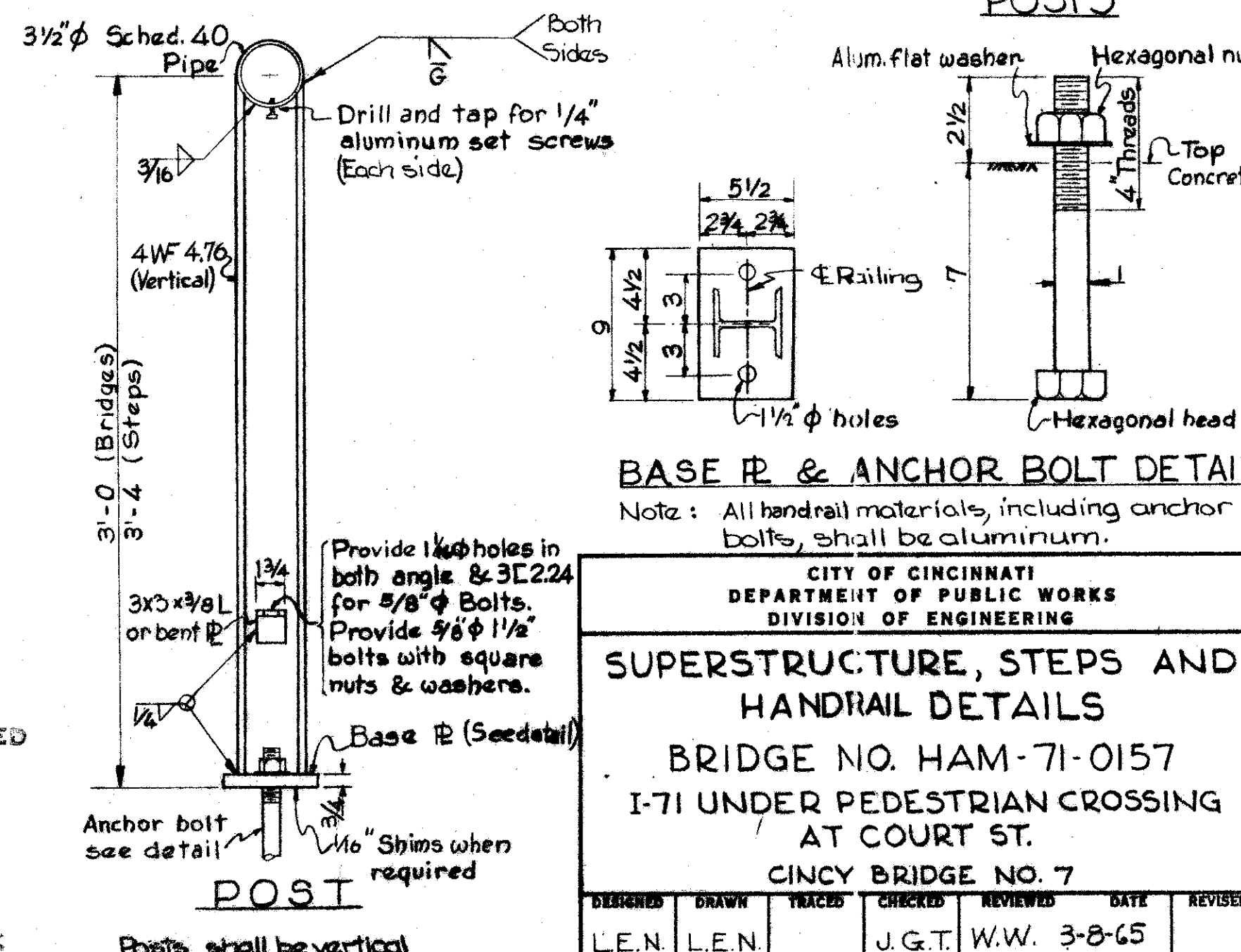
TYPICAL HANDRAIL CLOSURE BETWEEN POSTS



JOINT DETAIL



SECTION C-C



POST

BASE PL & ANCHOR BOLT DETAIL

Note: All handrail materials, including anchor bolts, shall be aluminum.

CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

SUPERSTRUCTURE, STEPS AND HANDRAIL DETAILS

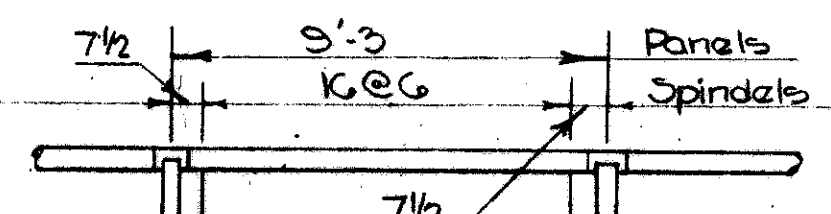
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.
CINCY BRIDGE NO. 7

DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
L.E.N.	L.E.N.	J.G.T.	W.W.	3-8-65	

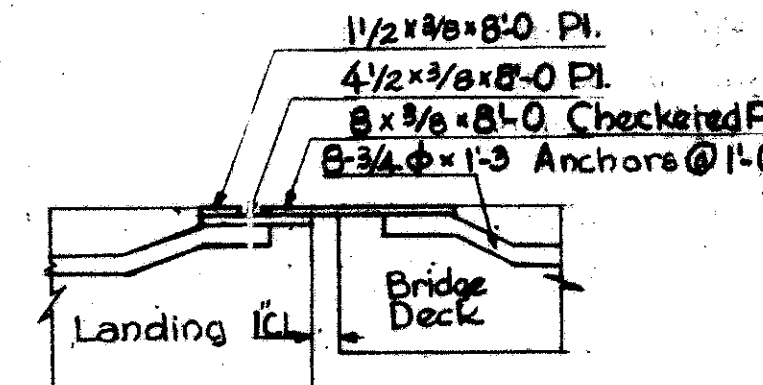
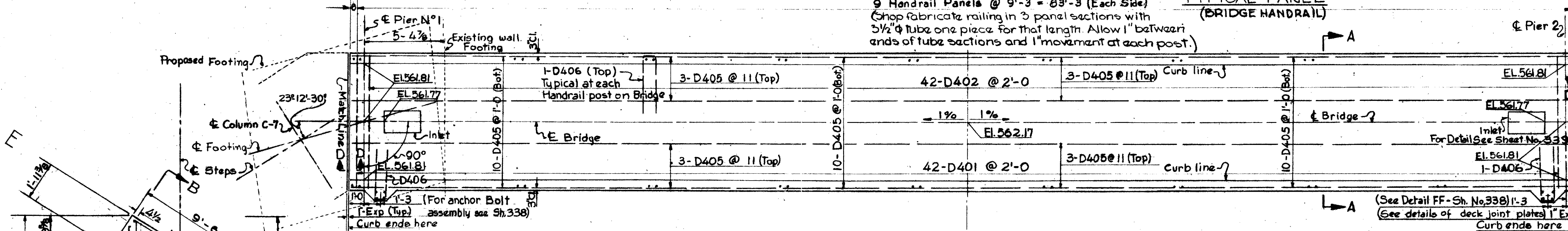
MICROFILMED
FEB 20 1985

This sheet superseded by
sheet No. 337A 9-27-66

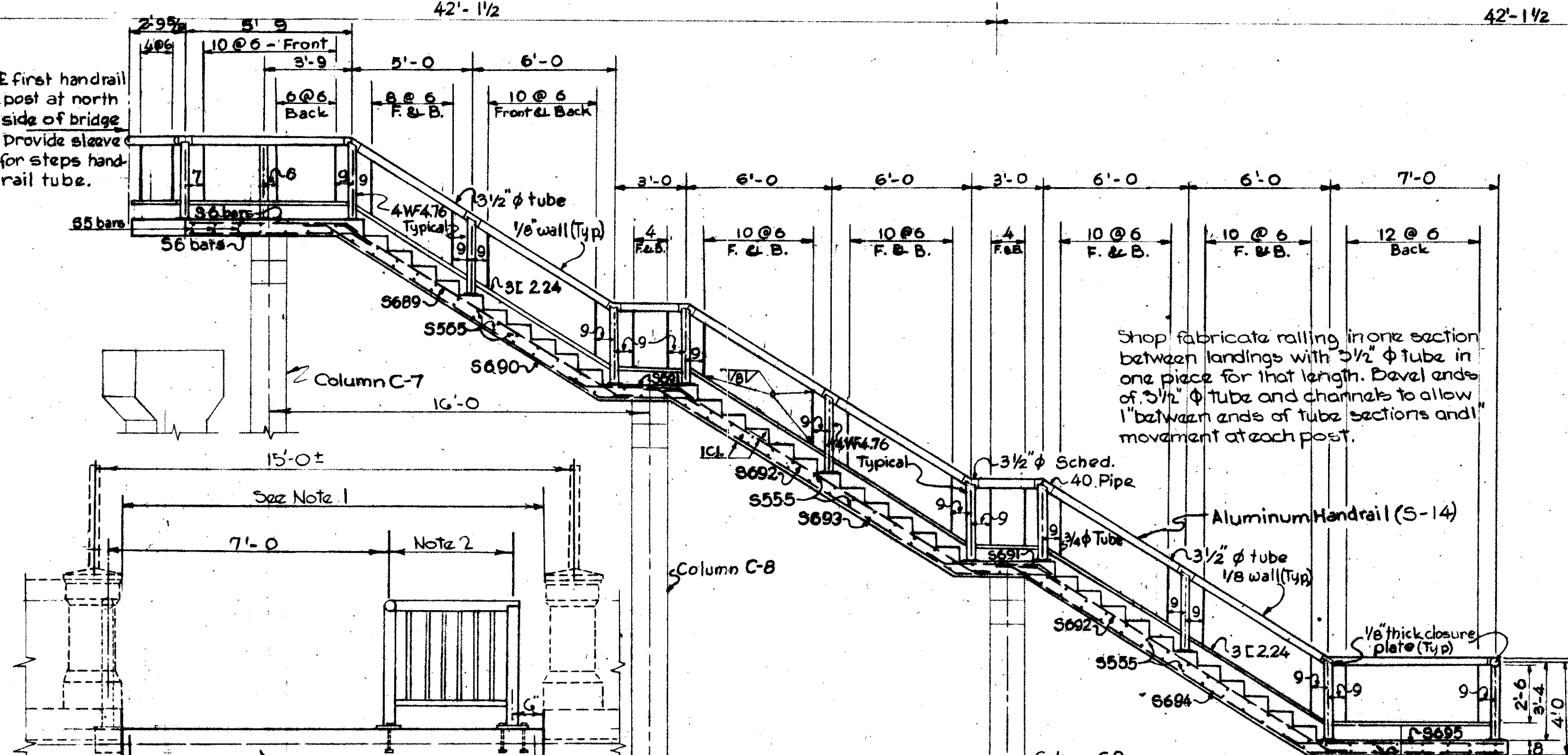
HAMILTON COUNTY
HAM-71-(15G)(2.51)



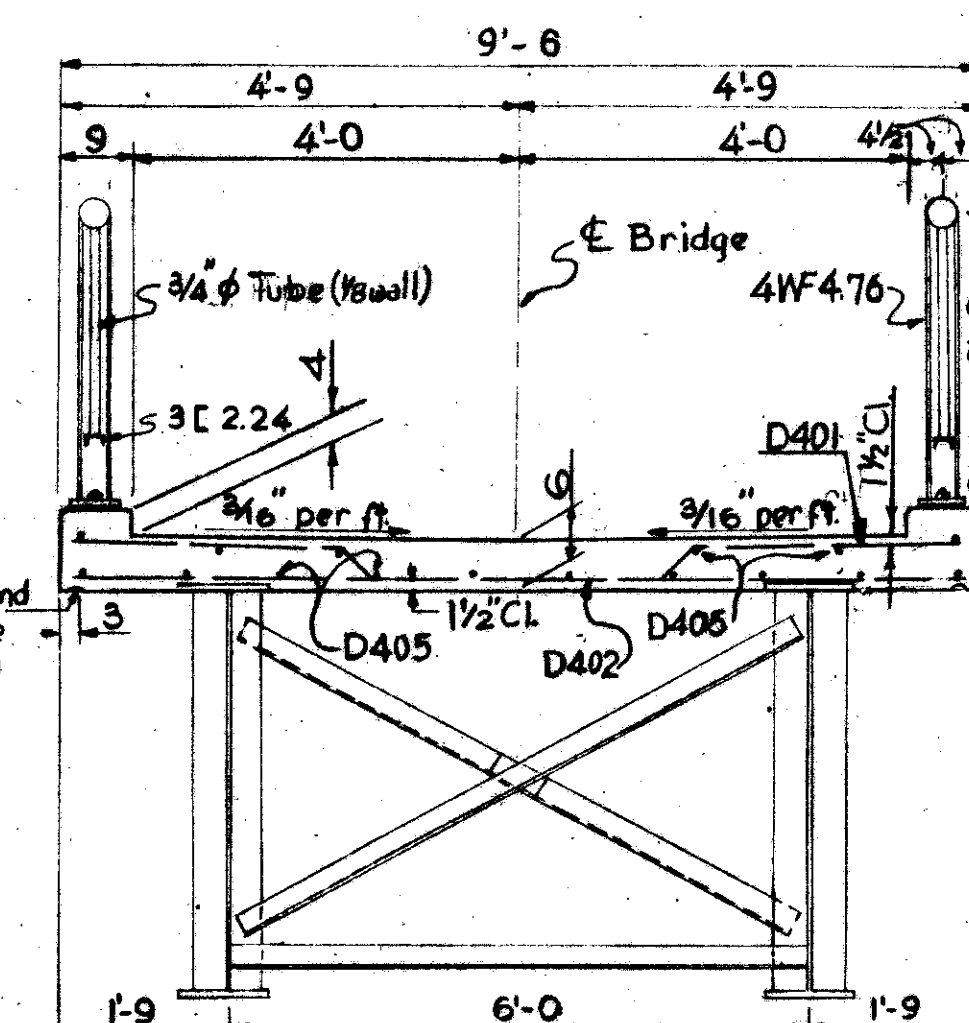
9 Handrail Panels @ 9'-3" = 83'-3" (Each Side)
Shop fabricate railing in 3 panel sections with 3/2" tube one piece for that length. Allow 1" between ends of tube sections and 1" movement at each post.



For details not shown see Joint Detail this sheet.
SECTION D-D



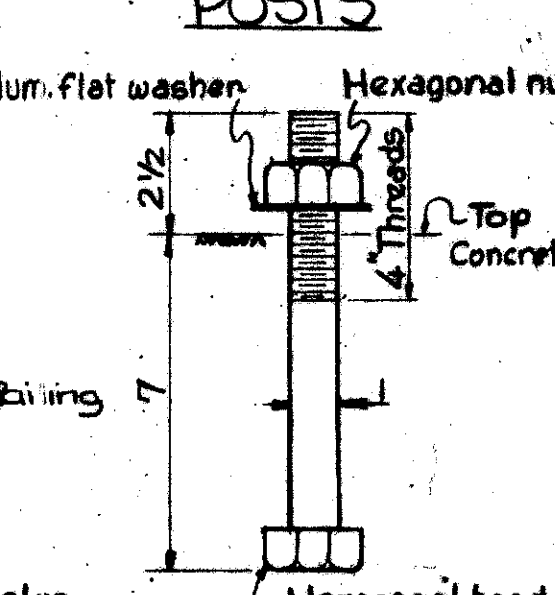
Shop fabricate railing in one section between landings with 3/2" tube in one piece for that length. Bevel ends of 3/2" tube and channels to allow 1" between ends of tube sections and movement at each post.



TYPICAL HANDRAIL DETAILS

SECTION A-A
For lighting Details, see sheet No.

TYPICAL HANDRAIL CLOSURE BETWEEN POSTS



BASE PL & ANCHOR BOLT DETAIL

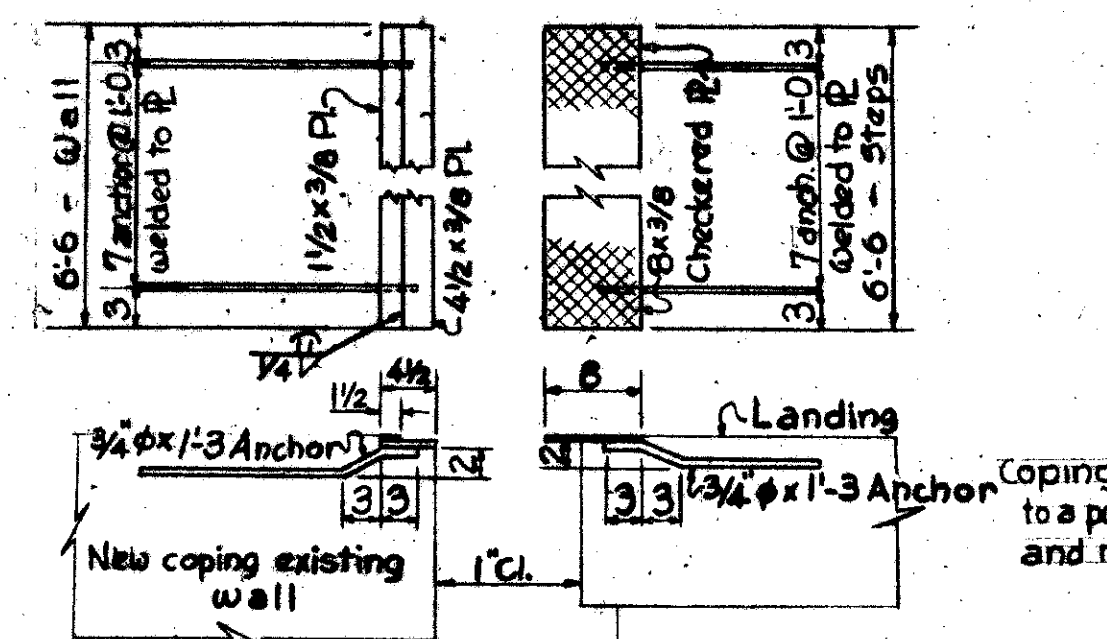
Note: All handrail materials, including anchor bolts, shall be aluminum.

CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING
SUPERSTRUCTURE, STEPS AND HANDRAIL DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING AT COURT ST.
CINCY BRIDGE NO. 7

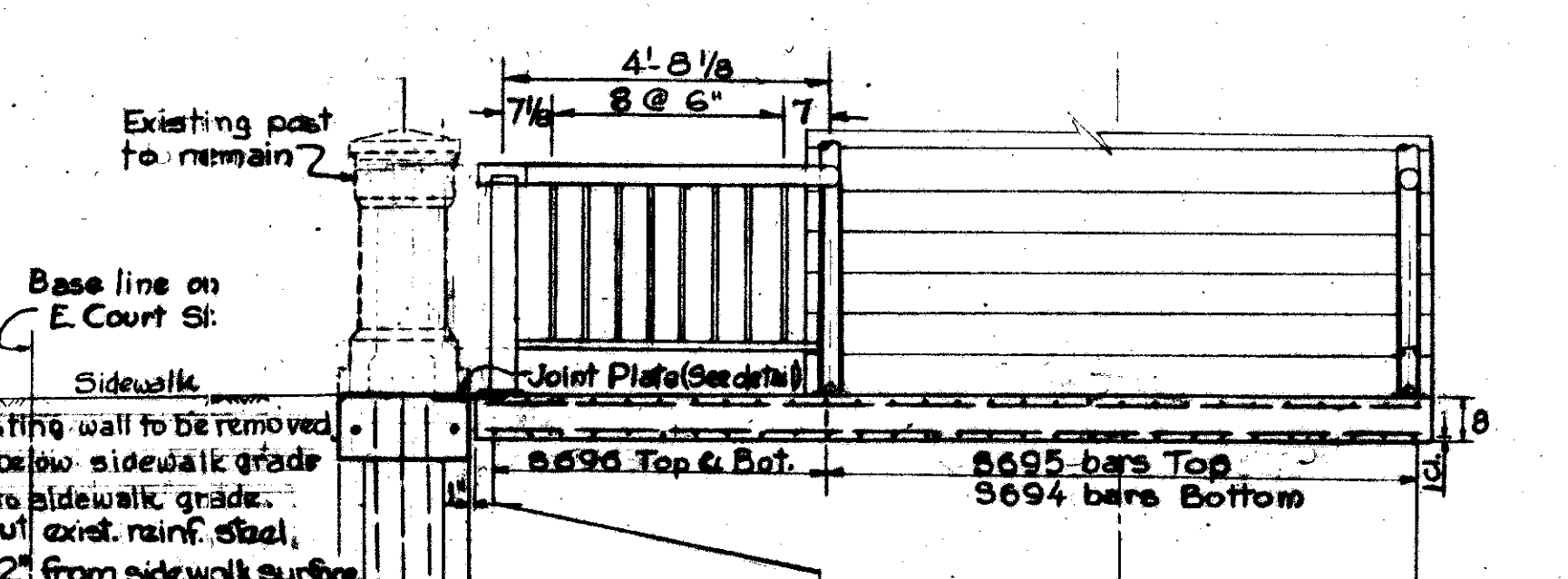
DESIGNED	DRAWN	CHECKED	APPROVED	DATE
L.E.N.	L.E.N.	J.G.T.	W.W.	3-8-65

Note 1
The existing railing, wall coping and fence between posts shall be removed completely. Provide fence terminal at posts. Cut existing horizontal reinf. bars 1" behind cut concrete surfaces and refinish as directed by Engineer.
Note 2: Posts on wall coping shall be lined up with step railing as shown. Aluminum railing panel dimensions shall then be determined for fabrication with a nominal spindle c-c spacing not to exceed 6".

Wall & Coping Items:
Removal, S-22
Replacement, S-1 (Superstructure and Steps).
Fencing Items:
Removal of existing fencing between posts (Note 1) and providing terminal. Connections shall be included in cost bid for Item I-26.



Existing reinforcement to remain as in Section F-F

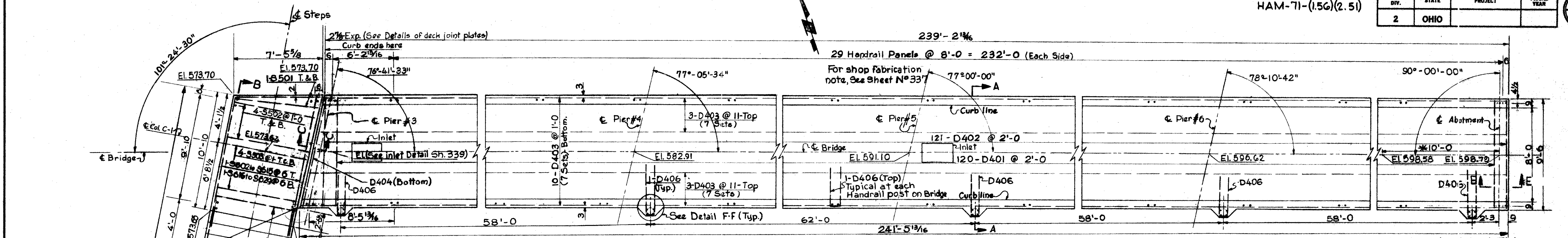


SECTION C-C
This sheet supersedes sheet No. 337 5-27-66

MICROFILMED
FEB 20 1965

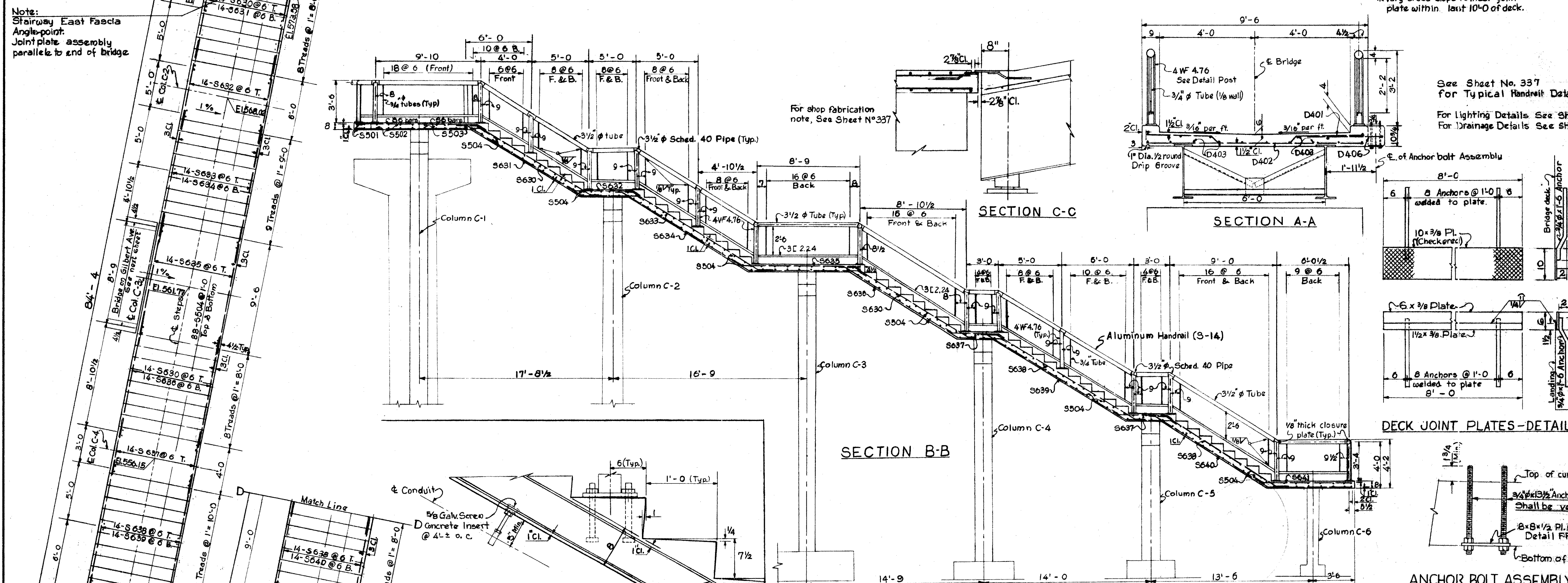
POST
Posts shall be vertical

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

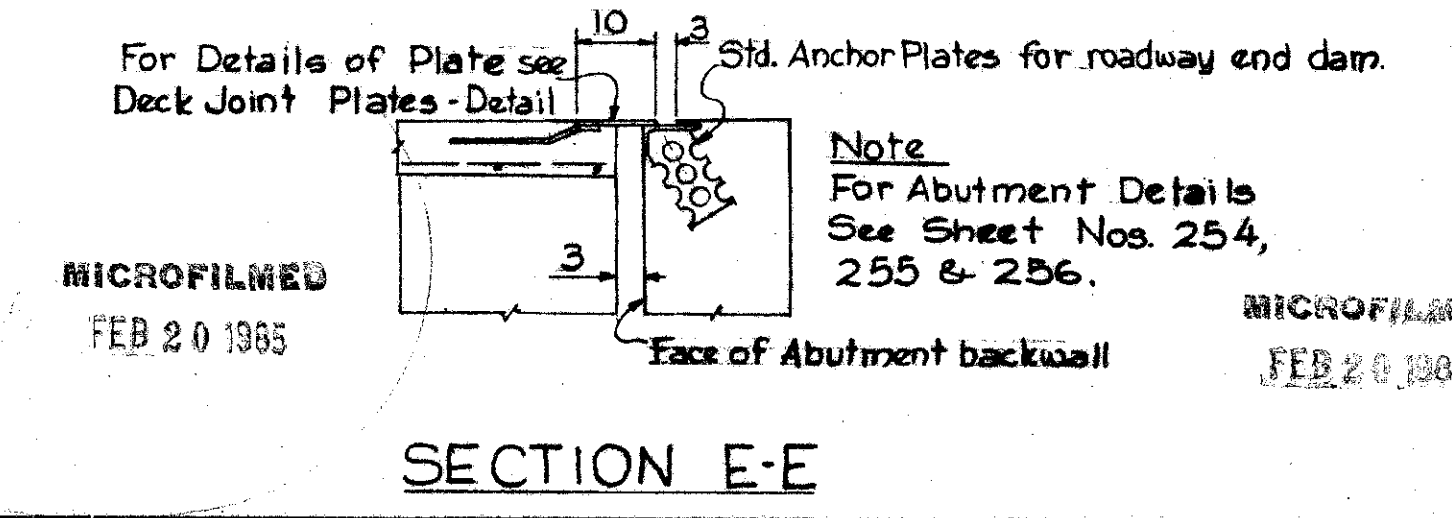
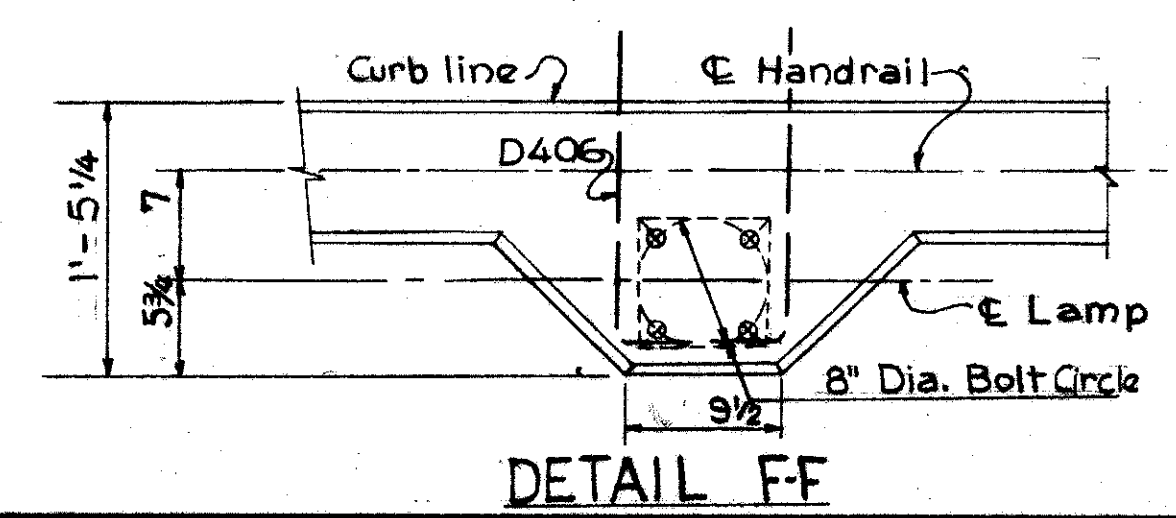
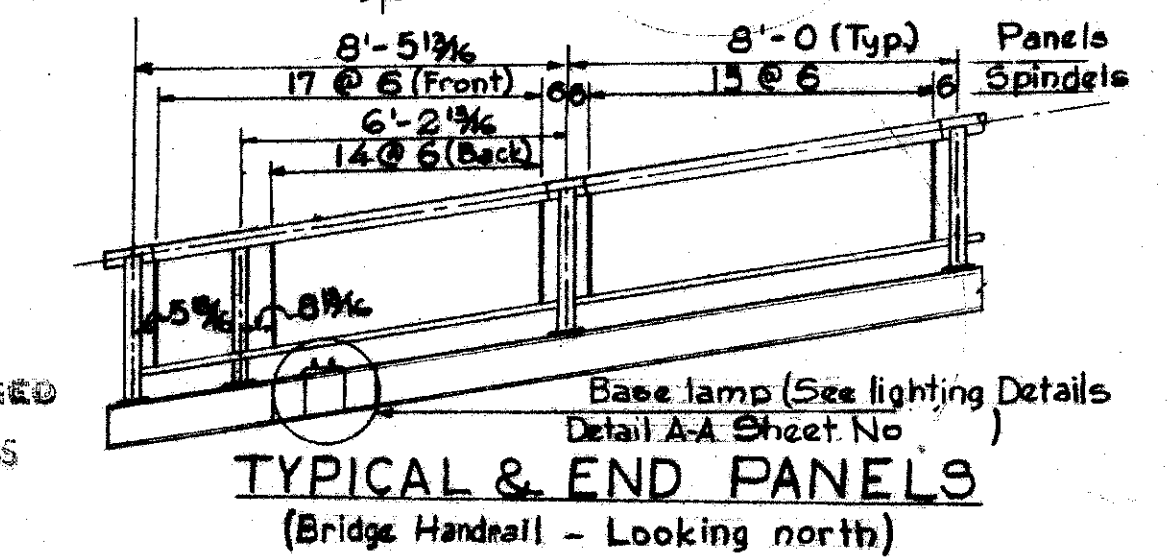
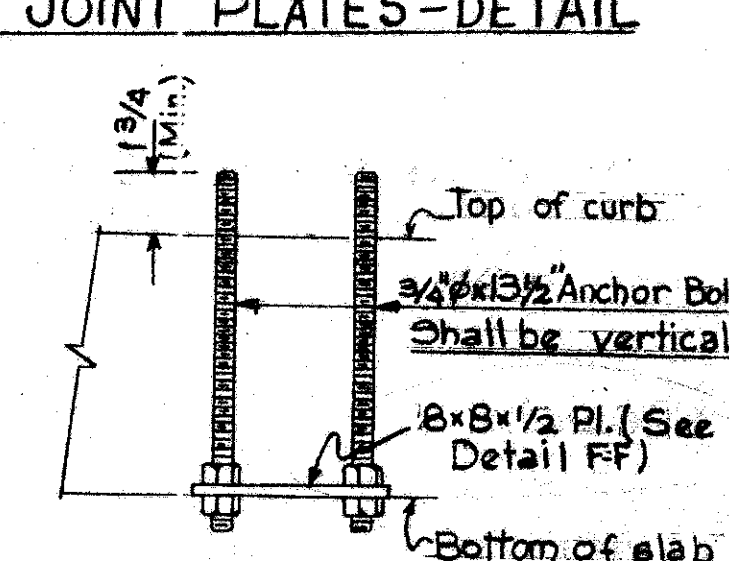
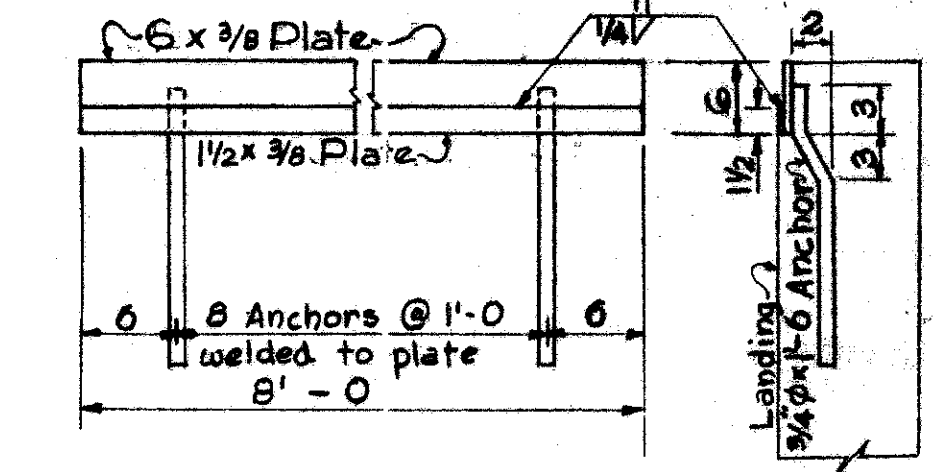
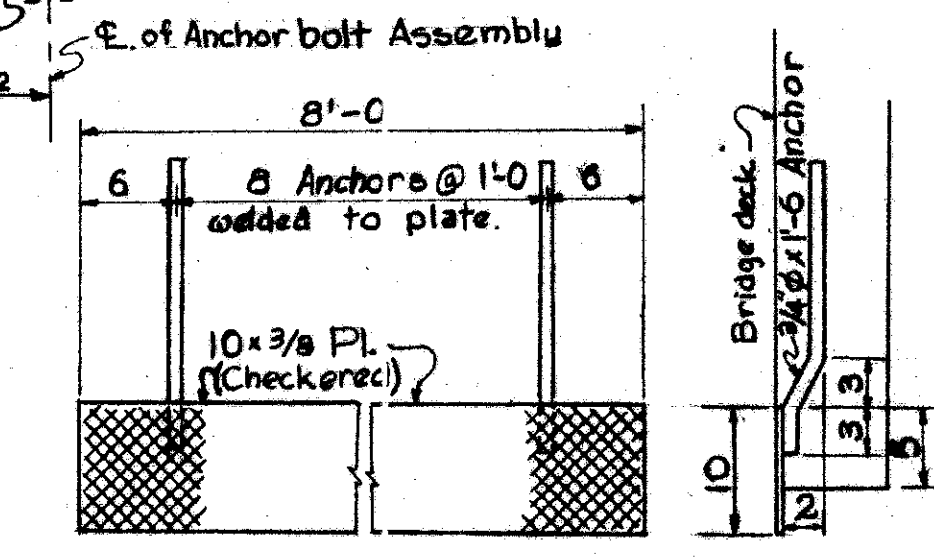


Note:
Stairway East Façia
Angle point
Joint plate assembly
parallel to end of bridge

*Vary cross-slope to meet joint plate within last 10'-0" of deck.



See Sheet No. 337
for Typical Handrail Details
For Lighting Details See Sh.
For Drainage Details See Sh. 339



CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

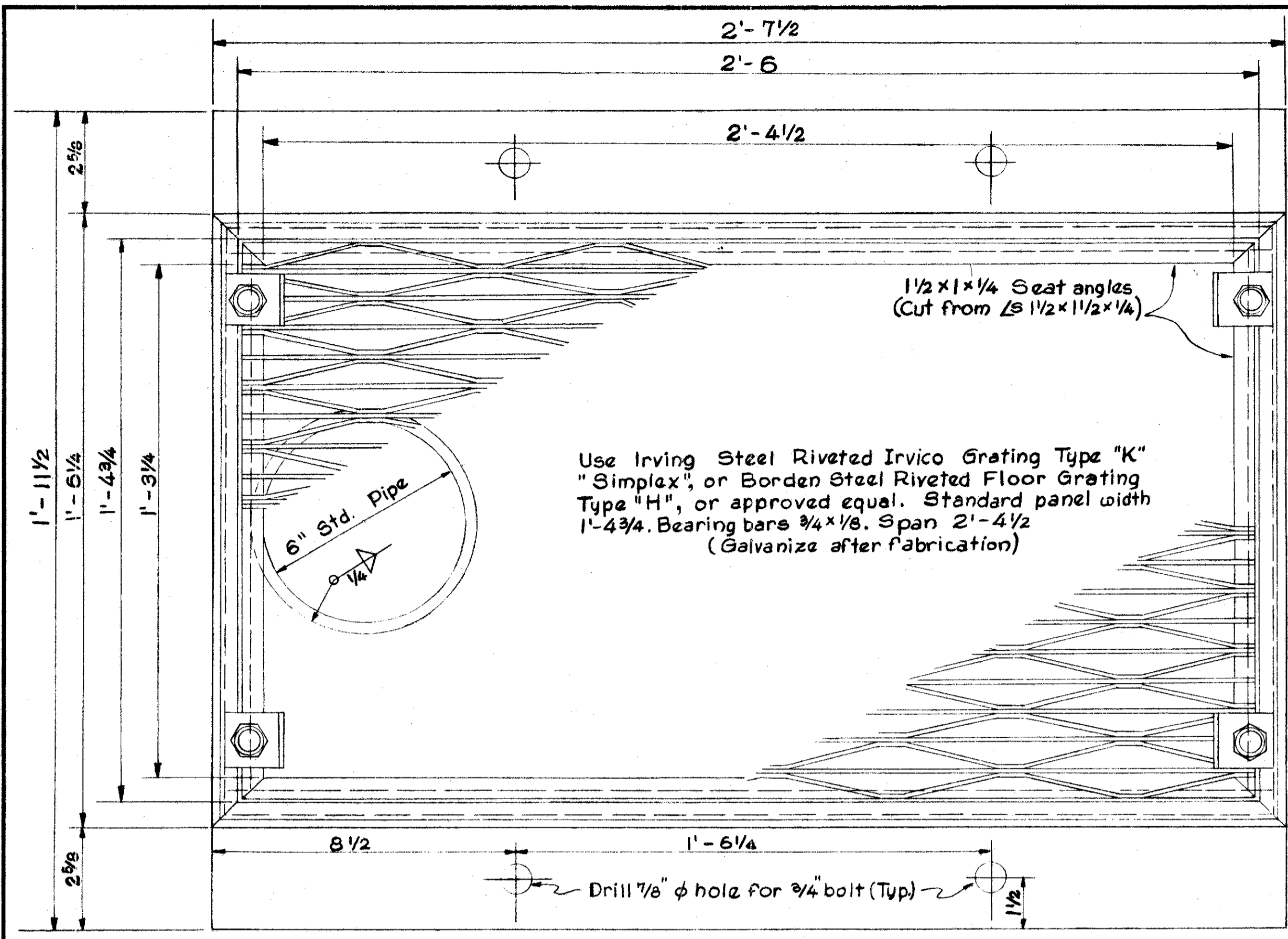
SUPERSTRUCTURE, STEPS AND HANDRAIL DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING AT COURT ST.
CINCY BRIDGE NO. 7

DRAWN	CHECKED	DATE
LEN	JGT	3-8-65

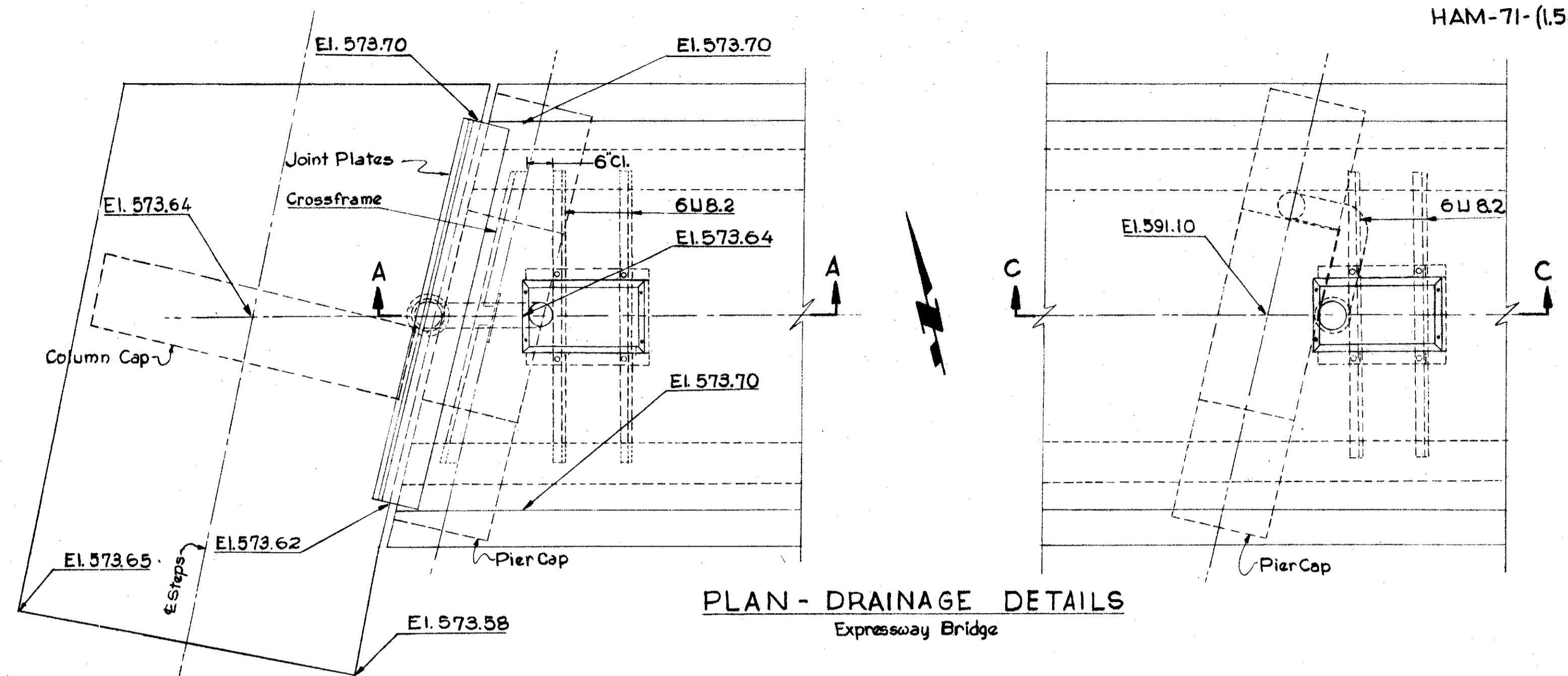
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

339
452

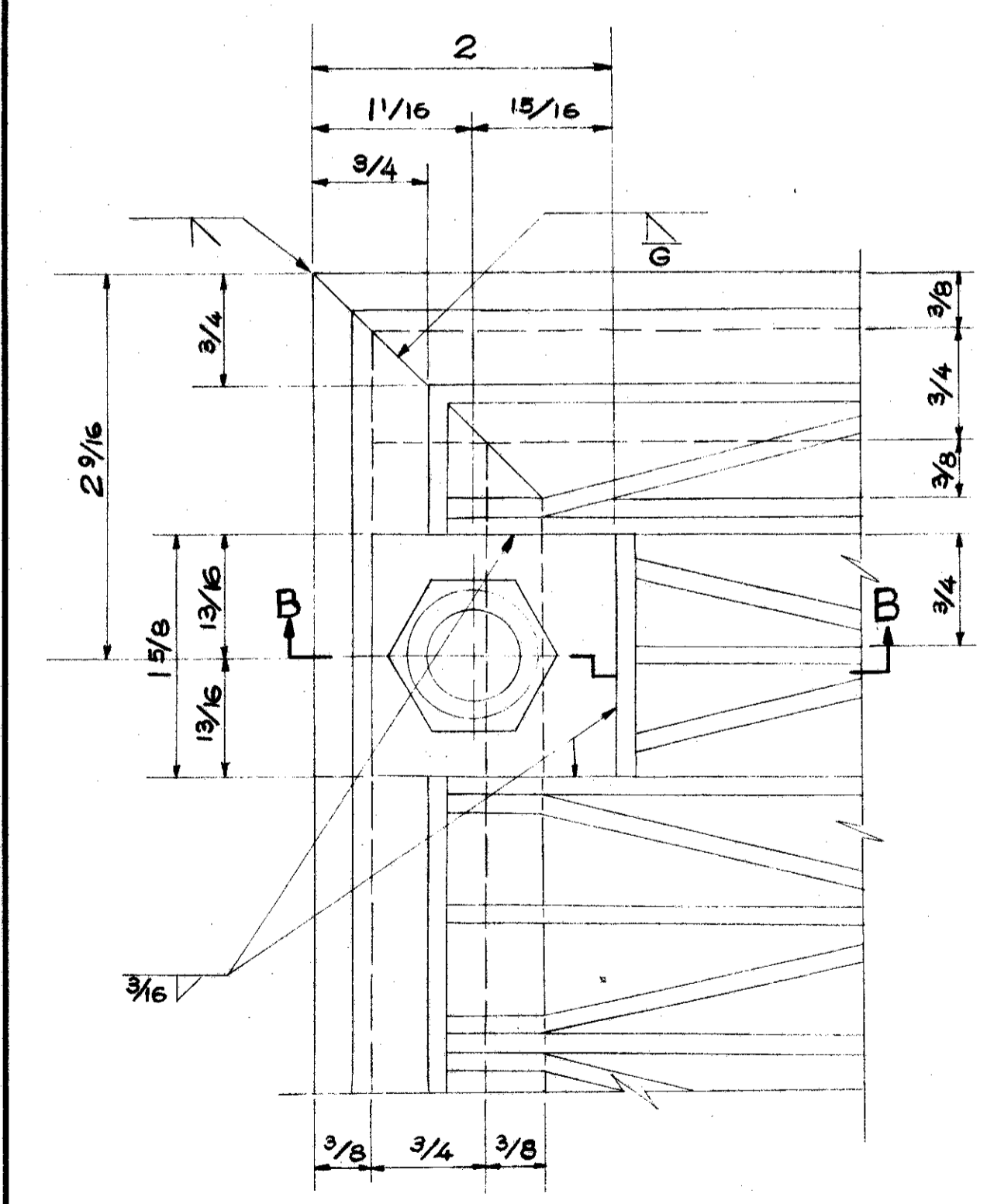
HAMILTON COUNTY
HAM-71-(1.56)(2.51)



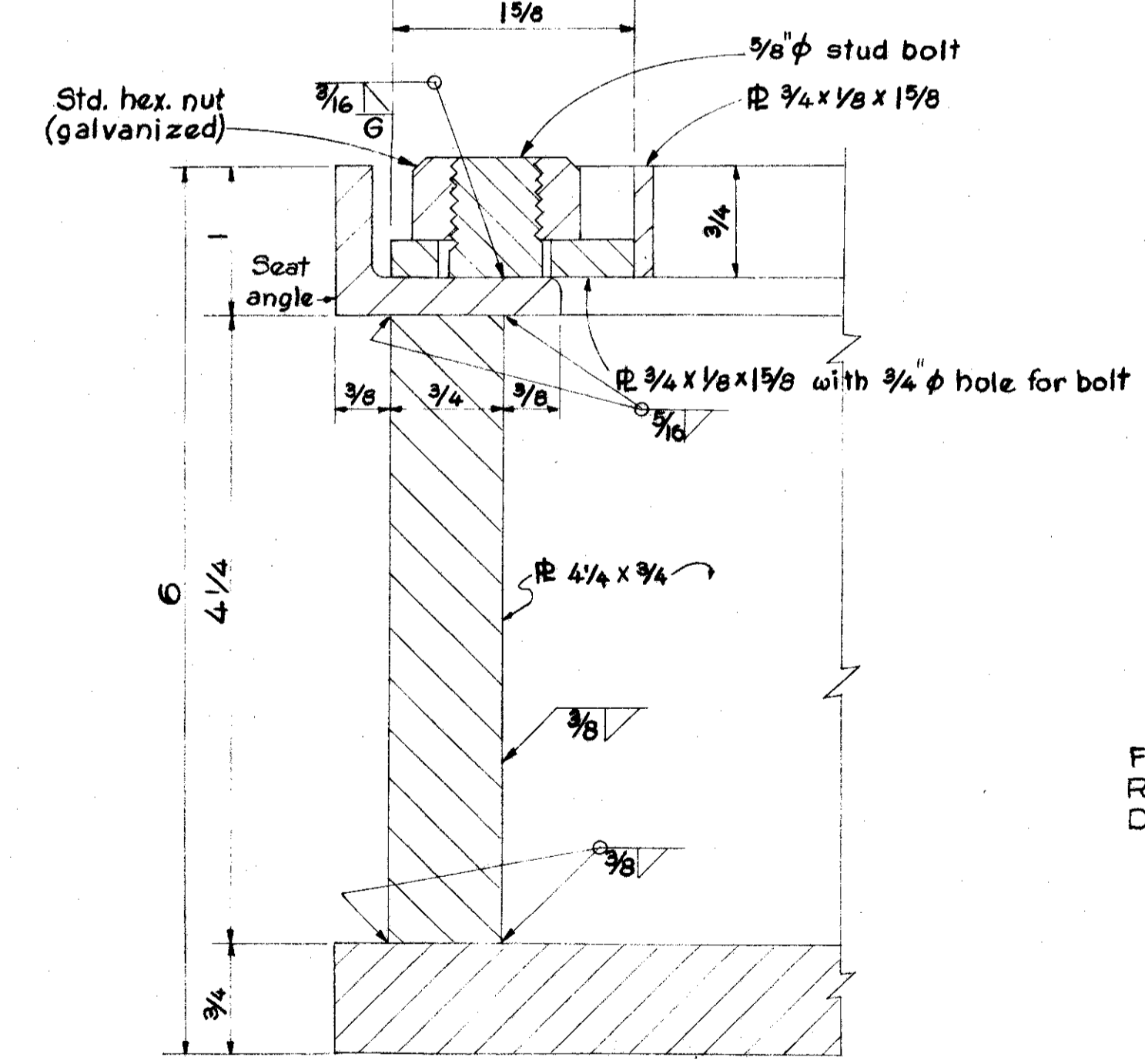
INLET FRAME & GRATING PLAN



PLAN - DRAINAGE DETAILS
Expressway Bridge



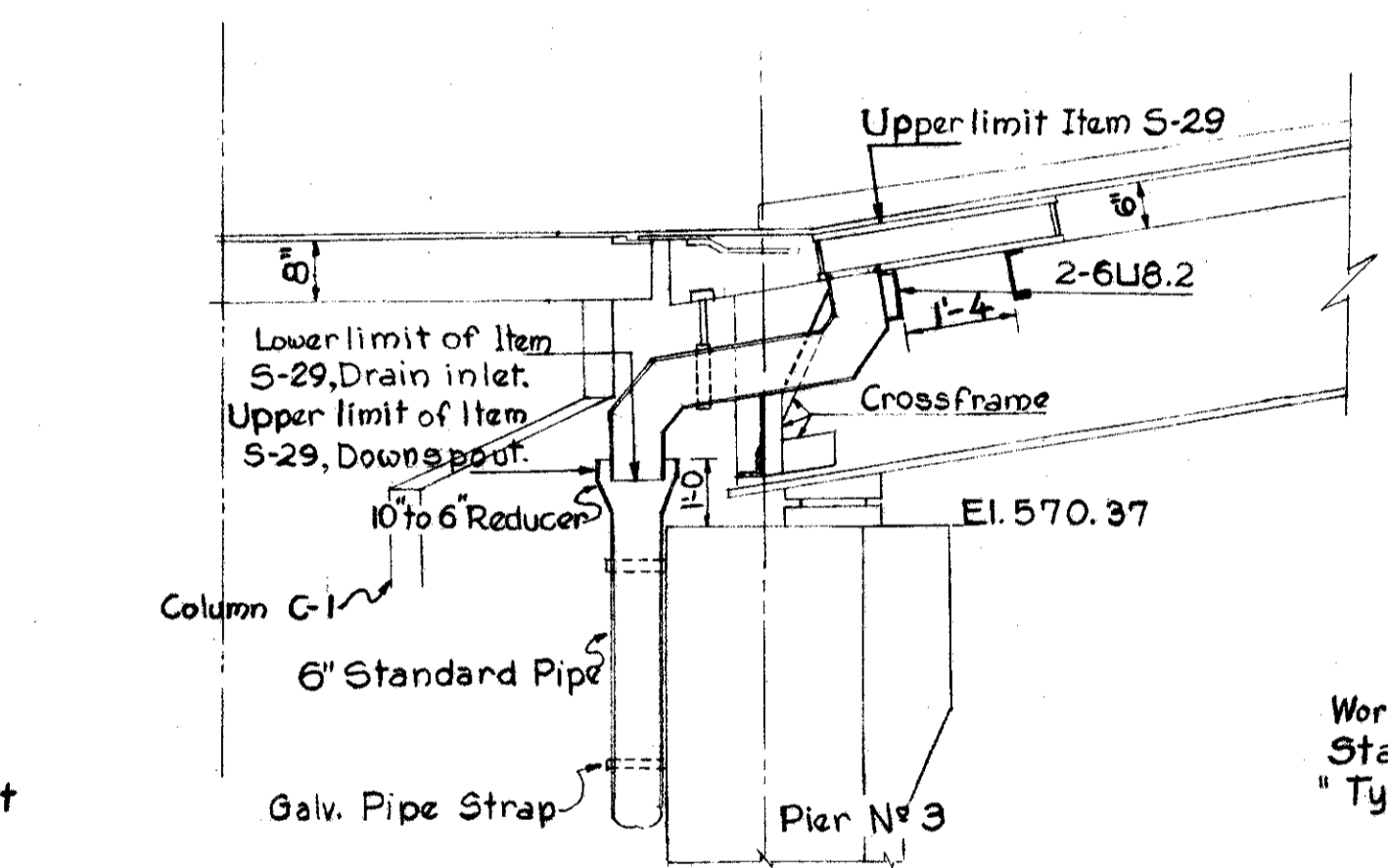
FASTENING DETAIL



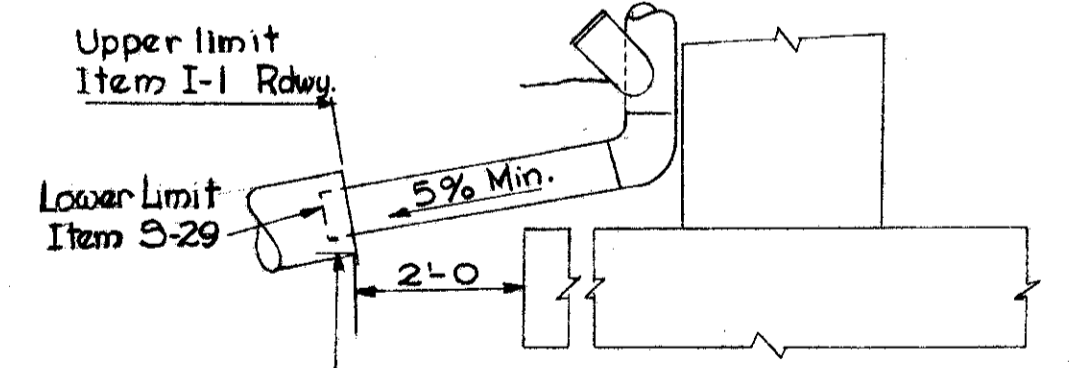
SECTION B-B

Note: For details of Inlet Frame not shown see Standard Drawing Sheet No. 445

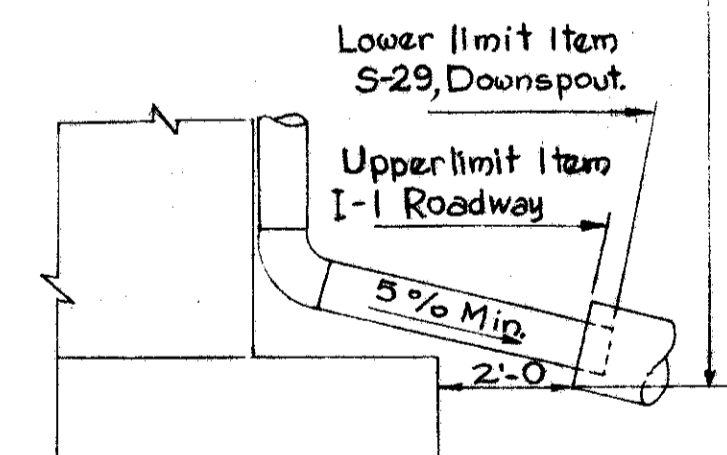
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SECTION A-A

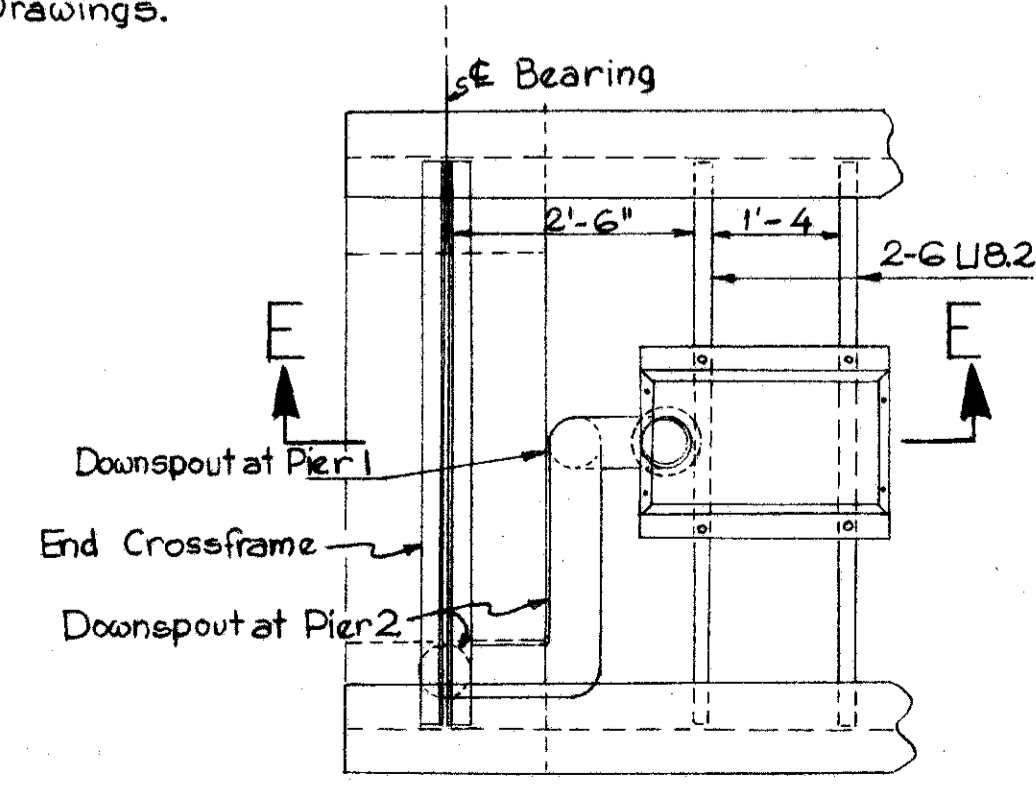


SECTION C-C

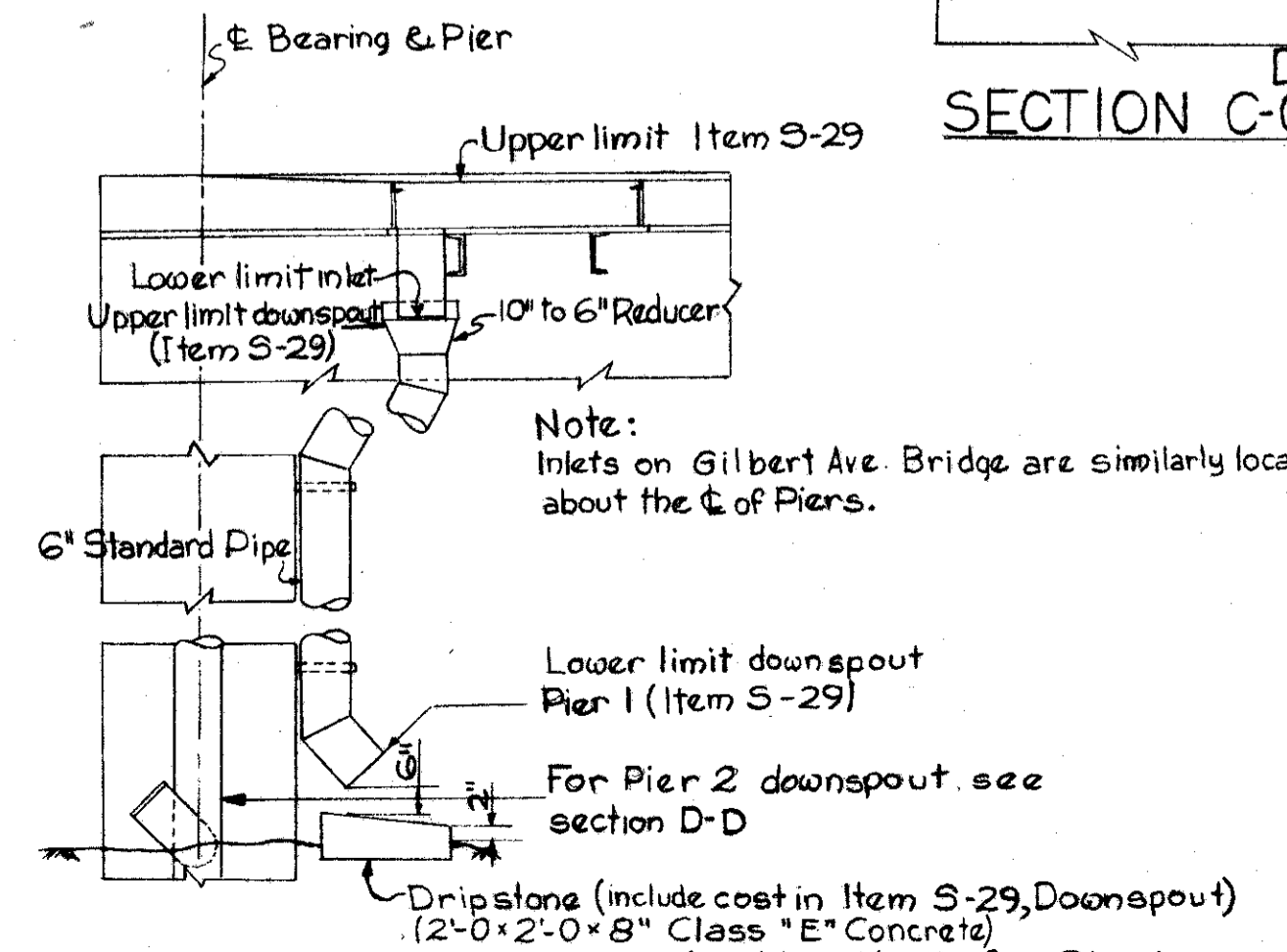


SECTION D-D

For Pier 5 Looking west
For Pier 2 Looking west



PLAN Gilbert Ave. Bridge



SECTION E-E

Note:
Inlets on Gilbert Ave. Bridge are similarly located about the C.C. of Piers.

For Pier 2 downspout see section D-D

Dripstone (include cost in Item S-29, Downspout)
(2'-0\"/>

Work this Drawing with
Standard Drawing Sheet No. 445
"Typical Drainage Details"

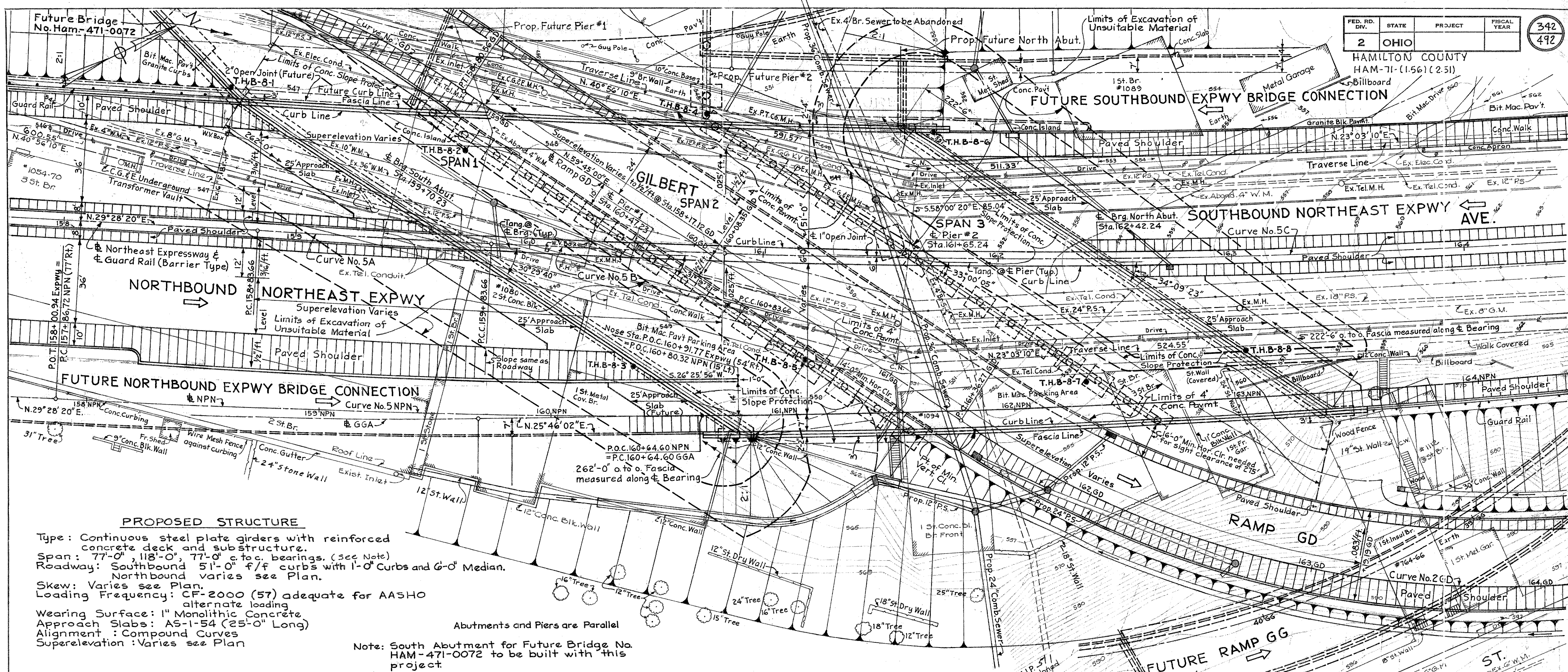
For elevation, see
Roadway Drainage
Drawings.

CITY OF CINCINNATI
DEPARTMENT OF PUBLIC WORKS
DIVISION OF ENGINEERING

DRAINAGE DETAILS
BRIDGE NO. HAM-71-0157
I-71 UNDER PEDESTRIAN CROSSING
AT COURT ST.

CINCY BRIDGE NO. 7

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE	REVISED
L.E.N.	L.E.N.		R.M.B.	W.W.	3-8-65	



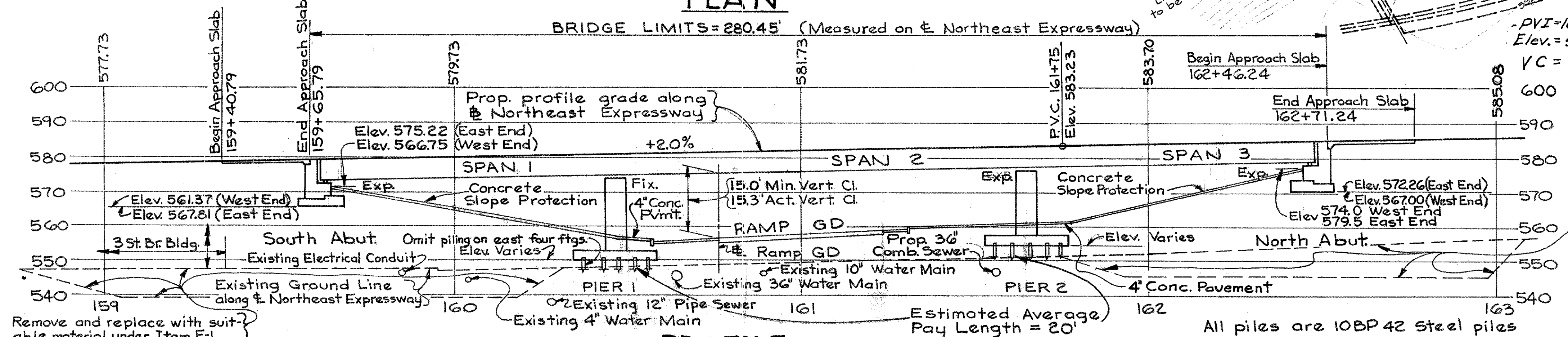
PROPOSED STRUCTURE

Type: Continuous steel plate girders with reinforced concrete deck and substructure.
 Span: 77'-0", 118'-0", 77'-0" c/c bearings. (See Note)
 Roadway: Southbound 51'-0" f/c curbs with 1'-0" Curbs and 6'-0" Median. Northbound varies see Plan.
 Skew: Varies see Plan.
 Loading Frequency: CF-2000 (57) adequate for AASHTO alternate loading
 Wearing Surface: 1" Monolithic Concrete
 Approach Slabs: AS-1-54 (25'-0" Long)
 Alignment: Compound Curves
 Superelevation: Varies see Plan

Note: South Abutment for Future Bridge No. HAM-471-0072 to be built with this project.

PLAN

BRIDGE LIMITS = 280.45' (Measured on E Northeast Expressway)



CURVE DATA

Curve No. 5A PI = 159+33.66 Δ = 00° 15' 01" Dc = 00° 15' 01" R = 22,892.88' L = 100.00' T = 50.00'	Curve No. 5B PI = 160+33.66 Δ = 01° 14' 59" Dc = 01° 14' 59" R = 4584.68' L = 100.00' T = 50.00'	Curve No. 5C PI = 166+27.38 Δ = 16° 12' 10" Dc = 01° 30' 00" R = 3819.72' L = 1080.19' T = 543.72'	Curve No. 5 NPN PI = 161+33.72 NPN Δ = 9° 13' 59" Dc = 01° 20' 00" R = 4297.19' L = 692.48' T = 346.99'
--	--	--	---

GENERAL NOTES:
 Span lengths are measured along Base Line
 Symbol denotes drill hole
 For test boring data, see Sheet 1, 2 & 10 of 13
 For Bench Marks, see Sheet 40

Traffic Count ADT = 39800 (NB) 25800 (SB)
 DIHV = 4610 (NB) 3450 (SB)

Remove and replace with suitable material under Item E-1

SITE PLAN

BRIDGE No. HAM-71-0173

Sta. 159+40.79 to Sta. 162+46.24

H&E BRIDGE No. 8R & L

HAZELIT & ERDAL
 CONSULTING ENGINEERS
 CINCINNATI, OHIO

DESIGNED: M.K.K. DRAWN: R.E.M. TRACED: J.H.D. CHECKED: J.H.D. REVISION DATE: 5/20/65

MICROFILMED
 FEB 20 1985

HAMILTON COUNTY
HAM-71-(156)(251)

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	SUPER-STRUCTURE	NORTH ABUTMENT	SOUTH ABUTMENT	PIER 1	PIER 2	GENERAL
E-2	2547	Cu. Yds.	Unclassified Excavation, including Rock (or Shale)		574	554	928	491	
S-1	1085	Cu. Yds.	Class "C" Concrete, Superstructure	1085					
S-1	341	Cu. Yds.	Class "C" Concrete, Piers above Footings				185	156	
S-1	659	Cu. Yds.	Class "E" Concrete, Abutments above Footings		302.2	356.8			
S-1	630	Cu. Yds.	Class "E" Concrete, Footings		115.5	134.0	197.3	183.2	
S-3	81	Lin. Ft.	Waterproofing, Premolded Sealing Strip		34	47			
S-4	506488	Lbs.	Reinforcing Steel	317,811	21,574	24,292	142,811		
S-7	1,250,000	Lbs.	Structural Steel	1,250,000					
S-8	1,250,000	Lbs.	Field Painting of Structural Steel	1,250,000					
S-9	317	Sq. Ft.	1" Preformed Expansion Joint Filler (M-10.02, Type I)		147	170			
S-14	586	Lin. Ft.	Railing Type "A" (Aluminum Rail & Supports, Concrete Parapet)	533	34	19			
S-14	281	Lin. Ft.	Railing (Type I-15.11 Galvanized with Galvanized Steel Posts, Base Plate & Bolts)	281					
S-16*	Lump	Sum	First Test Pile						Lump
S-18	4480	Lin. Ft.	Steel Piles, 10BP42				1920	2560	
S-25			Electric Lighting System (see Sh. No. 207)						
S-29	465	Lin. Ft.	6" Helical Perforated Corrugated Metal Pipe [Sec. M-6.4(h)] (Including Specials)		214	251			
S-29	475	Lin. Ft.	6" Helical Non-Perforated Corrugated Metal Pipe [Sec. M-6.4(h)]		203	272			
S-29	Lump	Sum	Drain Inlets, Including Supports and Horizontal Collector System	Lump					
S-29	145	Lin. Ft.	8" Standard Pipe Downspout, Wrought Iron or Hot-Dip Galvanized Steel, (Including Specials)			31	62	52	
S-29	293	Cu. Yds.	Porous Backfill		133	160			
I-10	2,250	Sq. Yds.	Concrete Slope Protection		1400	850			
S-101	1085	Each	Water-Reducing, Set-Retarding Admixture	1085					

*See note sheet no. 375.

GENERAL NOTES

REFERENCE shall be made to Standard Drawing I-15 No. 2-A.

EXCAVATION QUANTITY includes the removal of fill material required for construction of the Abutments and Piers.

FOUNDATION BEARING PRESSURE: Abutment footings are designed for a maximum bearing pressure of 1.6 tons per Sq. Ft. Pier footings I-N, I-P, I-R and I-S are designed for a maximum bearing pressure of 5.0 tons per sq. ft.

PILES Shall be driven with a hammer of not less than 11,000 ft. lbs per blow to firm contact with shale. If the length of penetration is approximately equal to the depth of shale according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Sec. S-1805 is not less than the following value for a pile hammer of the indicated energy rating:

50 tons per pile using an 11,000 ft. lb hammer.
45 tons per pile using a 15,000 ft. lb hammer.

If the energy rating of the hammer is between the ratings as shown above, the required formula capacity shall be determined by interpolation. The design load is 35 tons per pile.

POROUS BACKFILL: 1'-6" thick, full length of abutments shall extend up to the underside of the approach slab or to the finished ground surface.

ADDITIONAL NOTES: For Additional Notes see Notes 1, 2, 3, 5, 6, 7, & 8, Sh. No. 443

Design Loading CF = 2000 (S7)
Concrete Class C - Basic unit stress 1333 psi.
Concrete Class E - Basic unit stress 1133 psi.
Structural Steel - ASTM A36 - Basic unit stress 20,000 psi (Except for piles, ASTM A7 and A373 steel not permitted)
Reinforcing Steel - ASTM A15, A16, A160. Deformed Intermediate or Hard Grade. Basic unit stress 20,000 psi. Except, spiral reinforcement may be plain, Structural Grade with basic unit stress of 18,000 p.s.i.

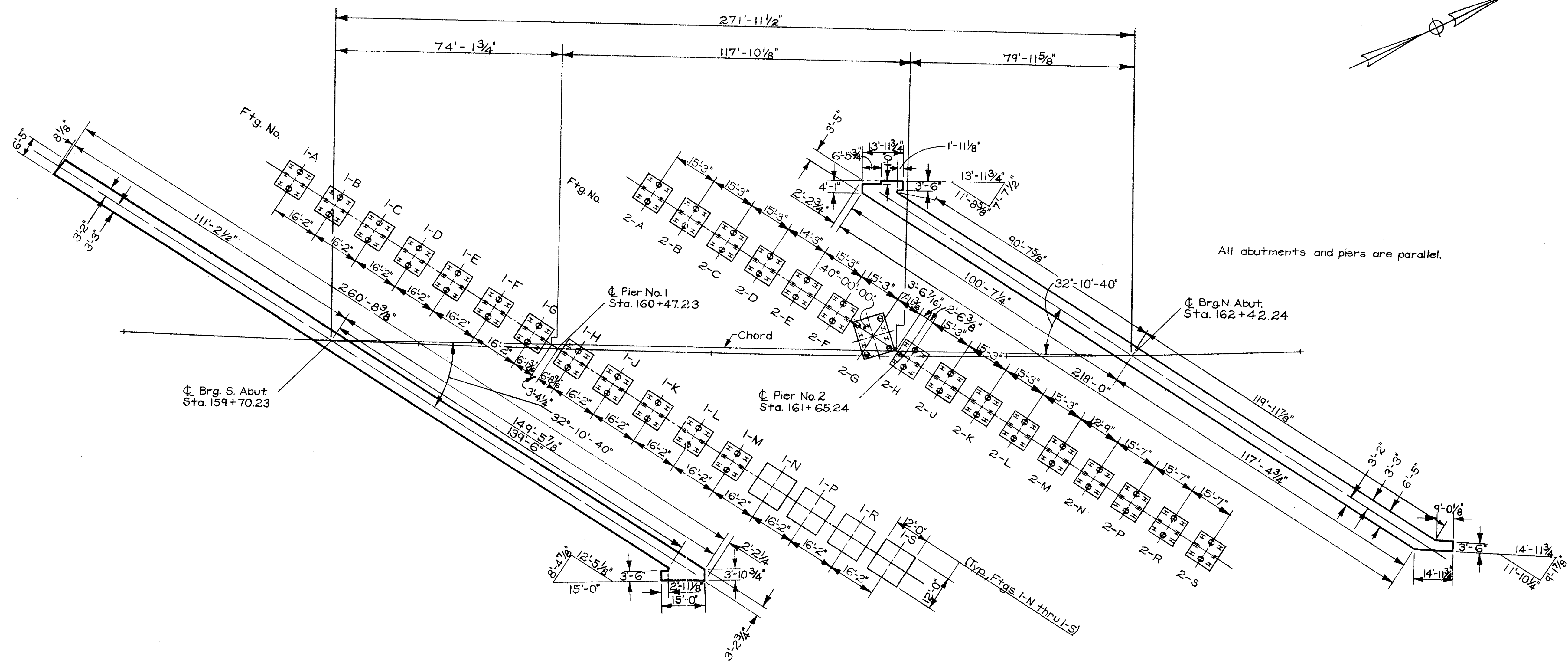
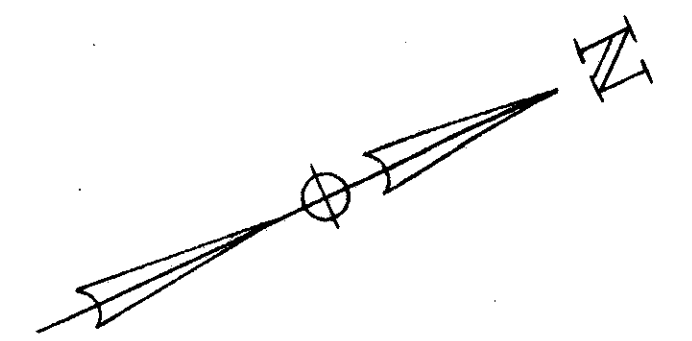
MICROFILMED
FEB 20 1965

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
ESTIMATED QUANTITIES AND GENERAL NOTES BRIDGE No. HAM.-71-0173				
H&E BRIDGE No. 8				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	CIB		JHD	5-20-65

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

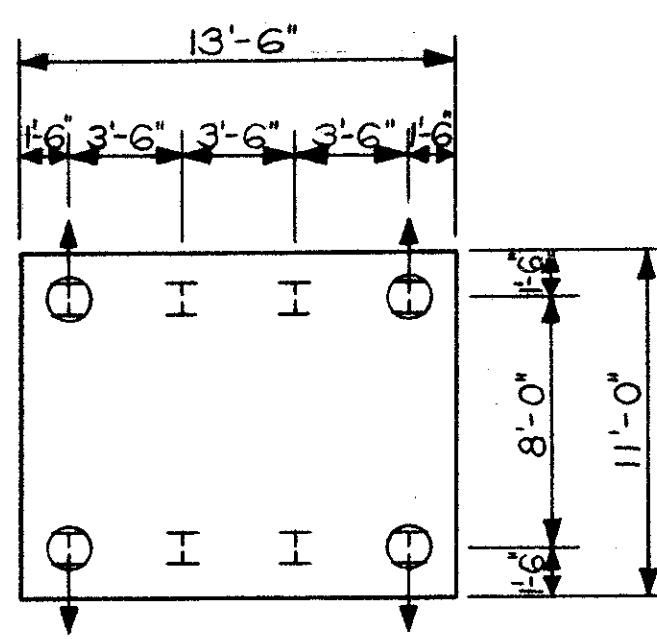
344
492

HAMILTON COUNTY
HAM-71- (1.56) (2.51)

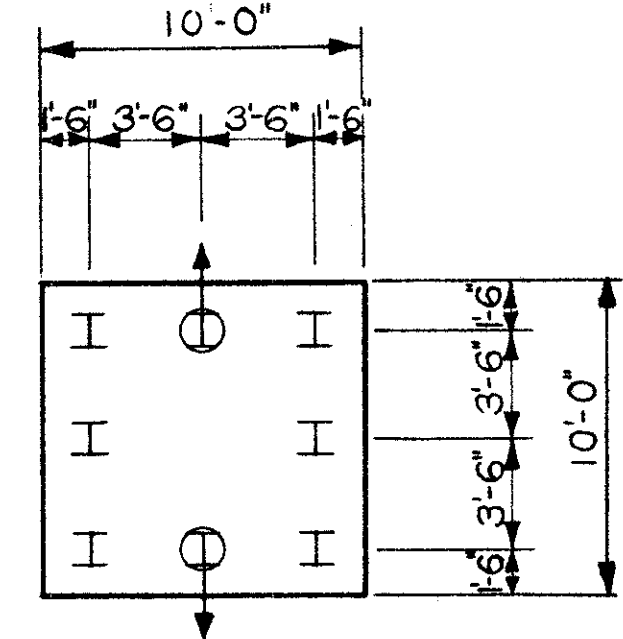


All piles are steel piles, 10BP42.
 Denotes battered piles (1 on 4).

PLAN



PLAN
FOOTING 2-G

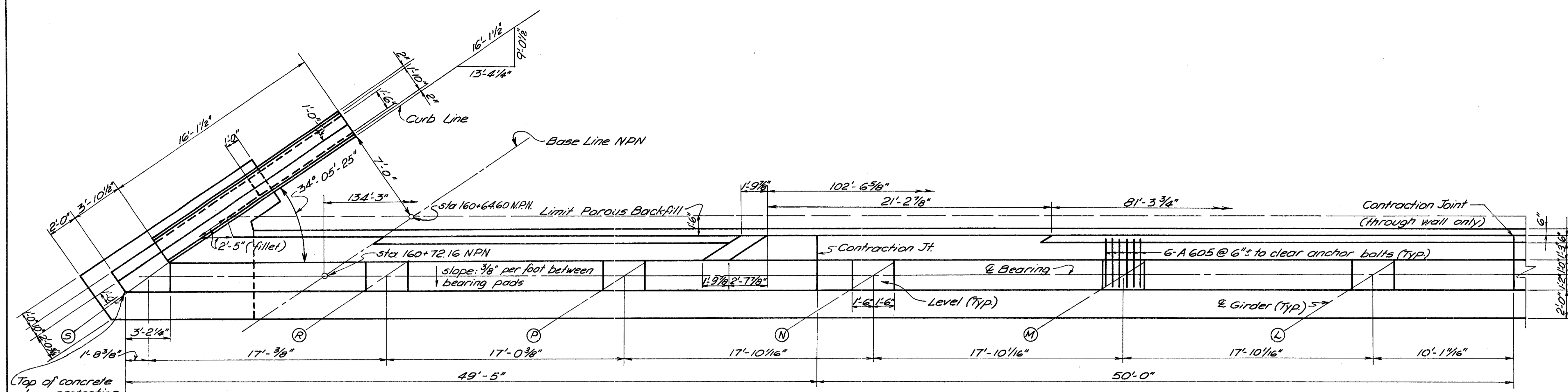
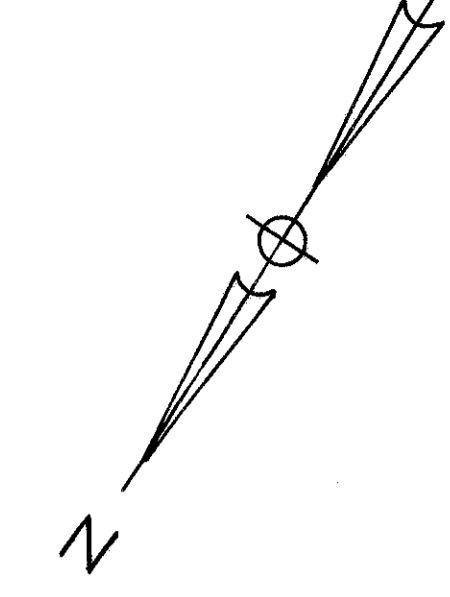


PLAN
ALL FOOTINGS EXCEPT
2-G & 1-N THRU 1-S

MICROFILMED
FEB 20 1965

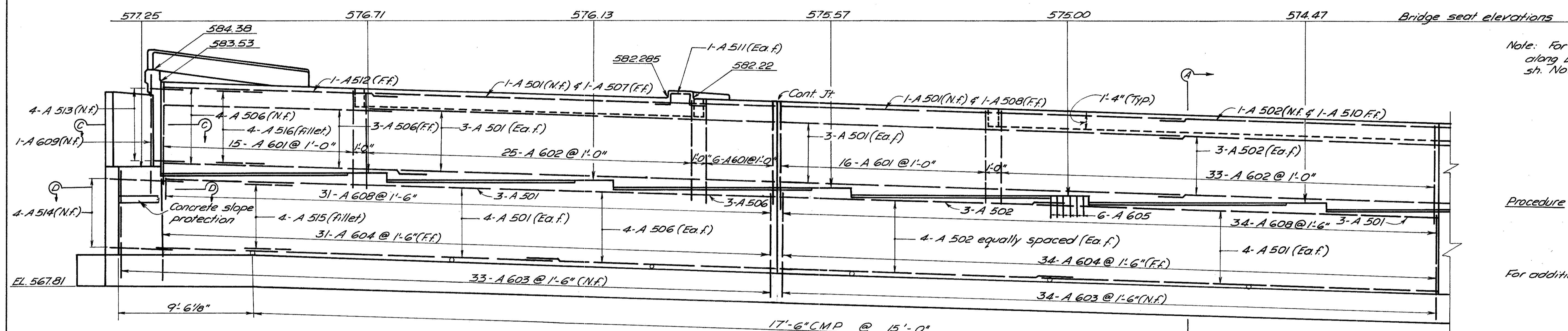
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STAKE-OUT-DIAGRAM BRIDGE No. HAM-71-0173					
H&E BRIDGE No. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	CIB		JHO	11/23/65	
				5-20-65	

HAMILTON COUNTY
HAM-71-(1.56) (2.51)



For lighting details See Sh. No. 214

PART PLAN

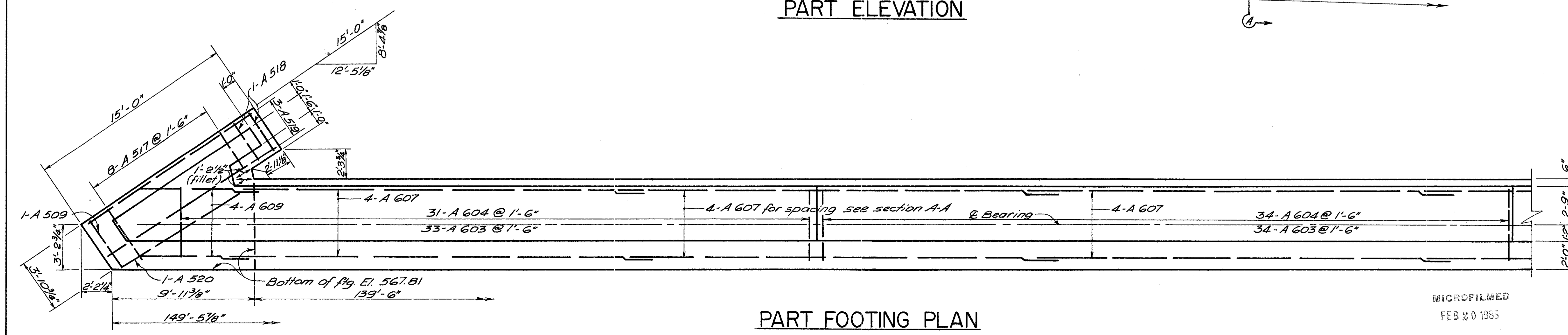


Note: For top of backwall elevations along backwall see End Finish Detail sh. No. 363

Procedure: For procedure note see sh. No. 349

For additional notes see sh. No. 350

PART ELEVATION



PART FOOTING PLAN

MICROFILMED
FEB 20 1965

work this sheet with sh. 346 thru 348

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SOUTH ABUTMENT
BRIDGE NO. HAM.-71-0173

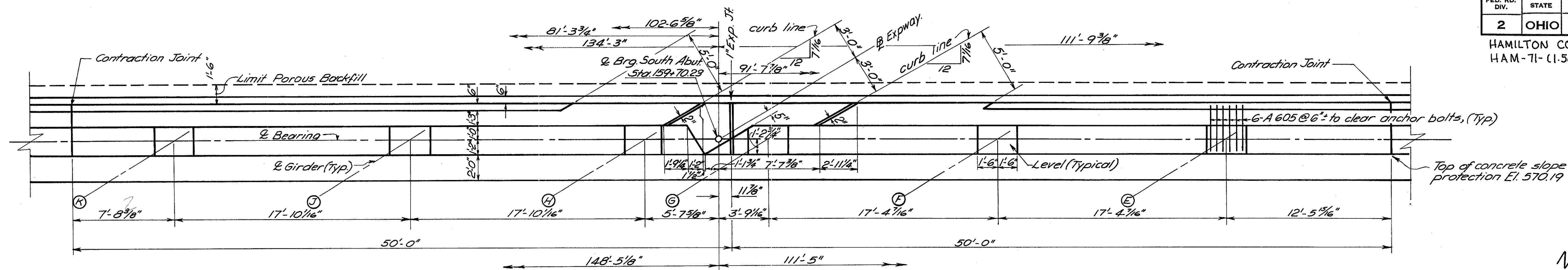
H&E BRIDGE NO. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	10-16-64		1-21-65	H.A.S.	
	A.N.	B.Sch.	J.H.O.	5-20-65	

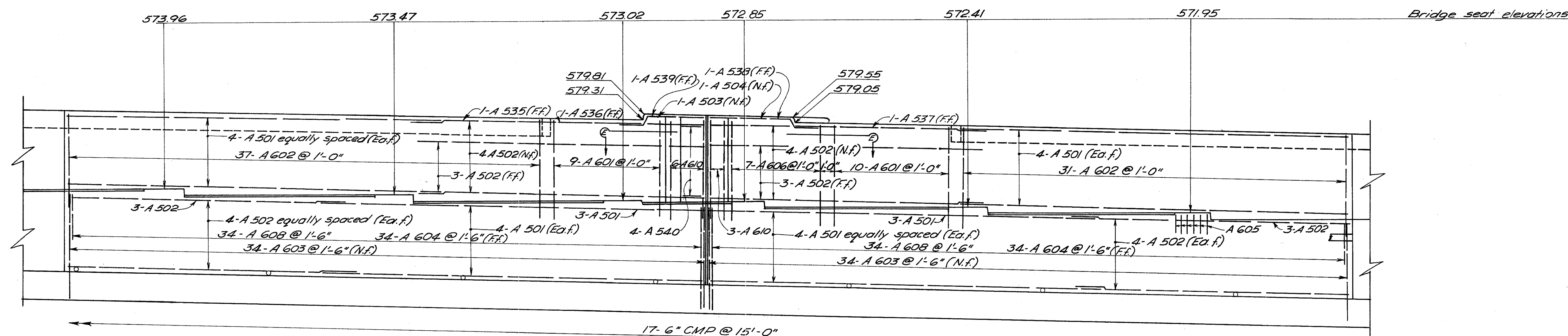
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

346
492

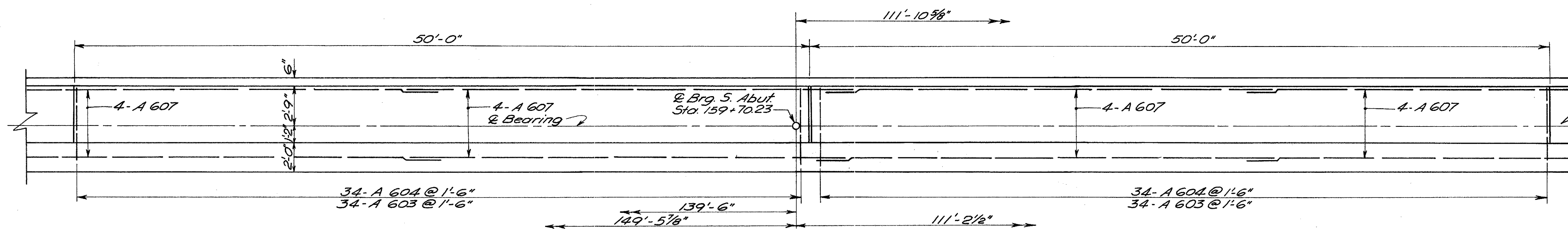
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



PART PLAN



PART ELEVATION

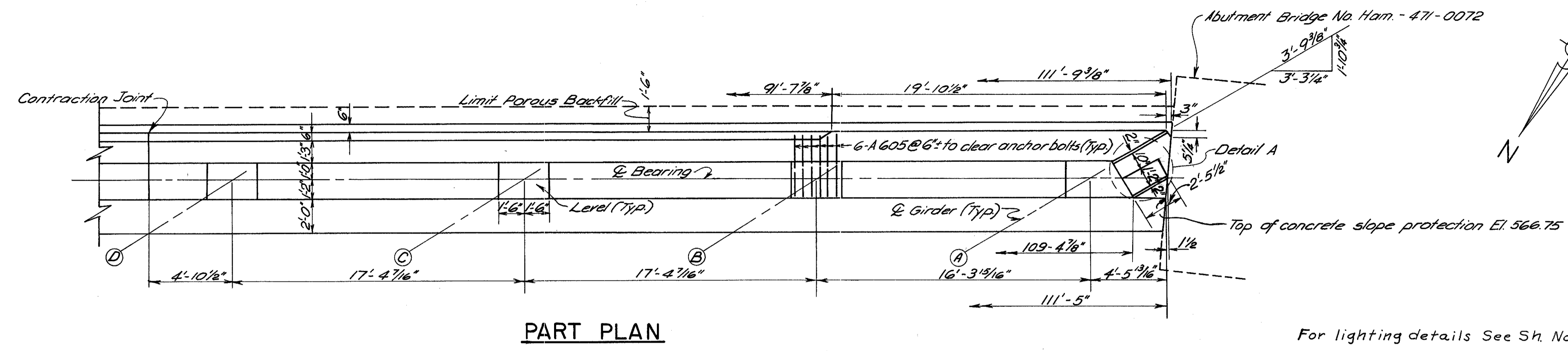


PART FOOTING PLAN

work this sheet with sheets 345, 347, & 348

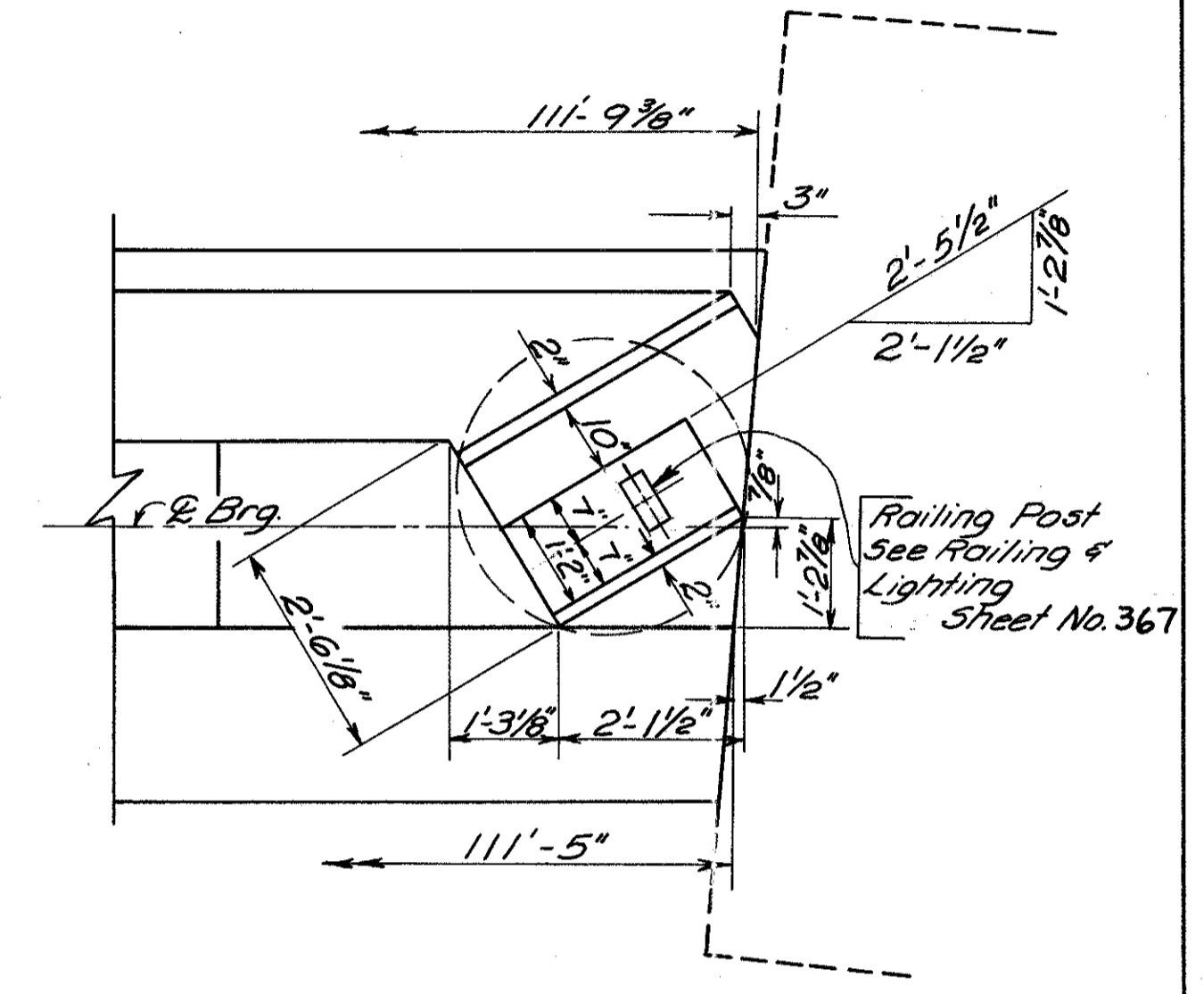
HAZELT & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
SOUTH ABUTMENT					
BRIDGE NO. HAM.-71-0173					
H & E BRIDGE NO. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
A.N.	B.Sch.	J.H.O.	H.N.S.	1-21-65	5-20-65

MICROFILMED
FEB 20 1965

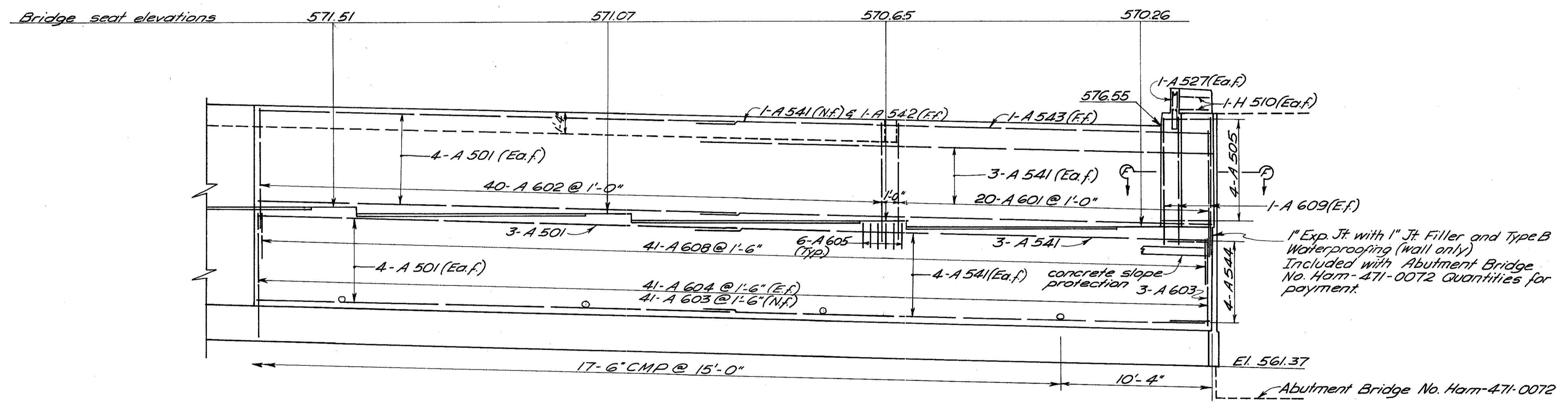


PART PLAN

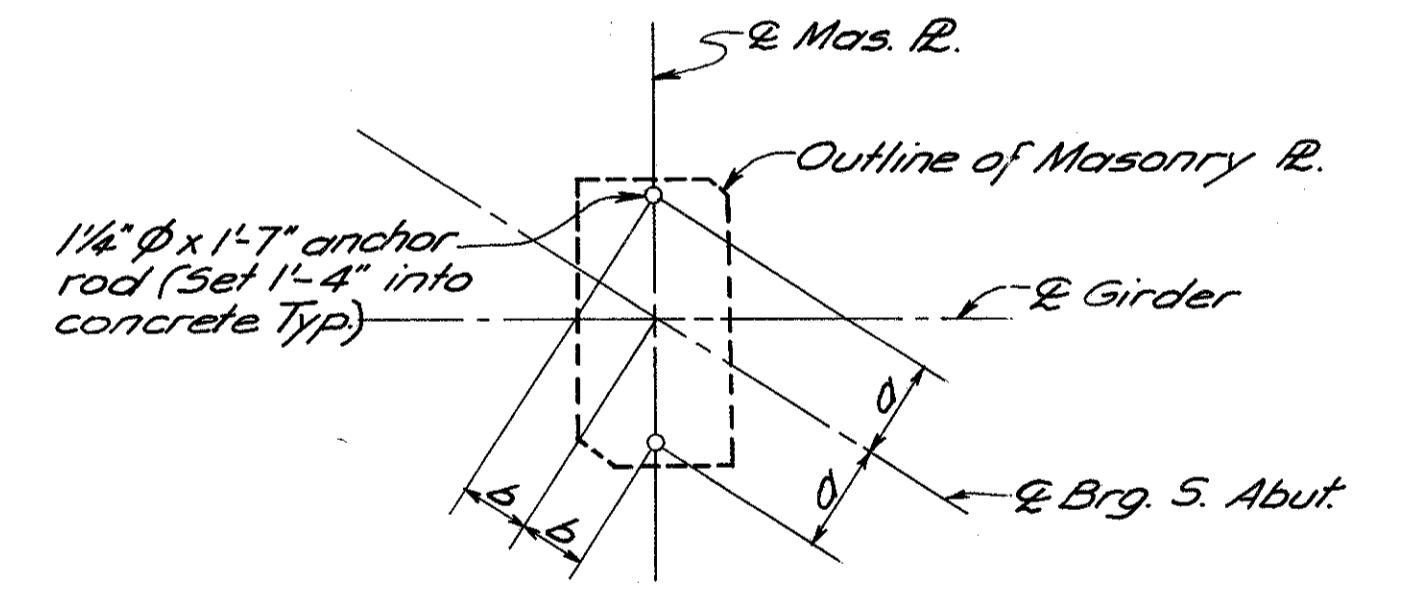
For lighting details See Sh. No. 214.



DETAIL A

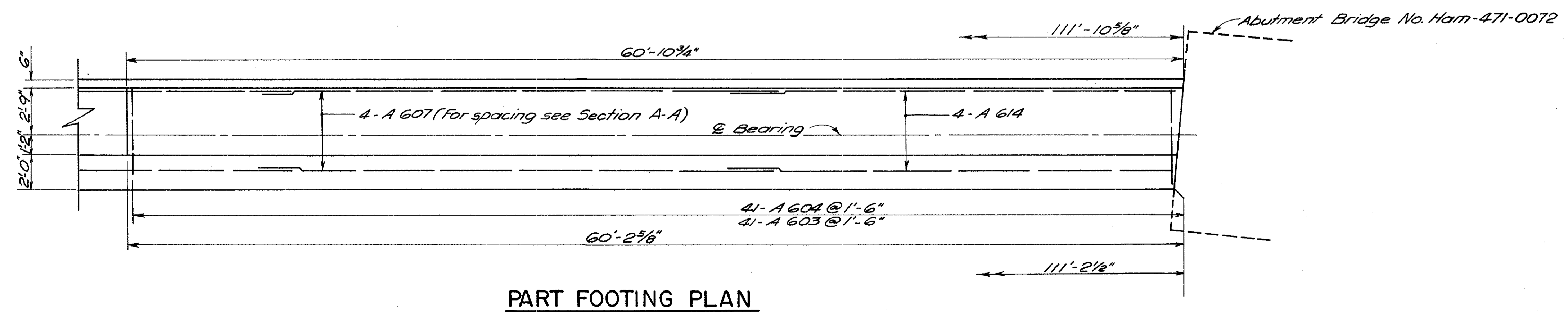


PART ELEVATION



ANCHOR ROD LAYOUT

Girder	a	b
A	8 5/8"	5 1/8"
B thru P	8 7/8"	5 3/8"
R	8 3/8"	5 3/8"
S	8 1/8"	5 7/8"



PART FOOTING PLAN

MICROFILMED
FEB 20 1985

work this sheet with sheets 345, 346 & 348

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

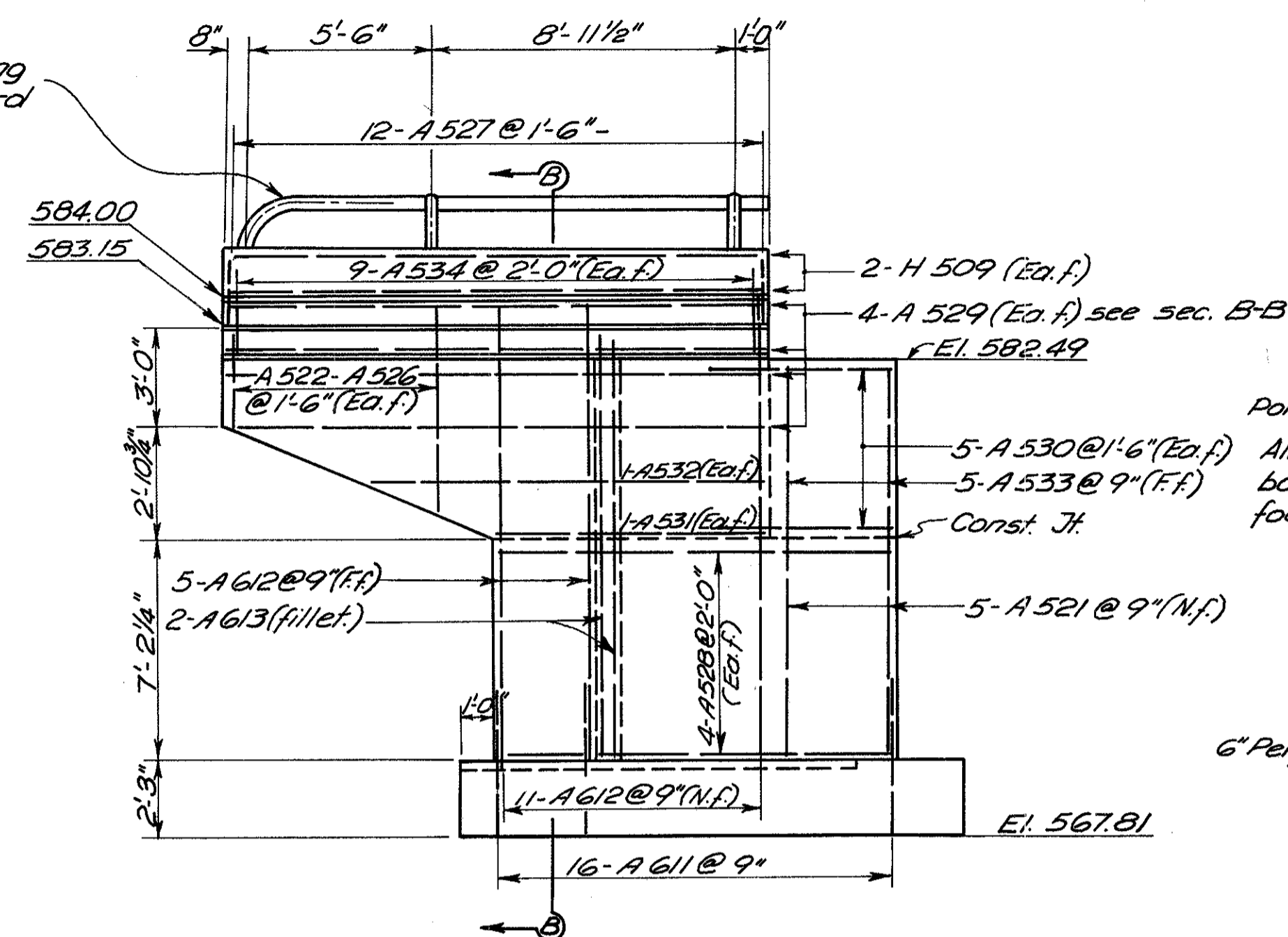
SOUTH ABUTMENT

BRIDGE NO. HAM.-71-0173

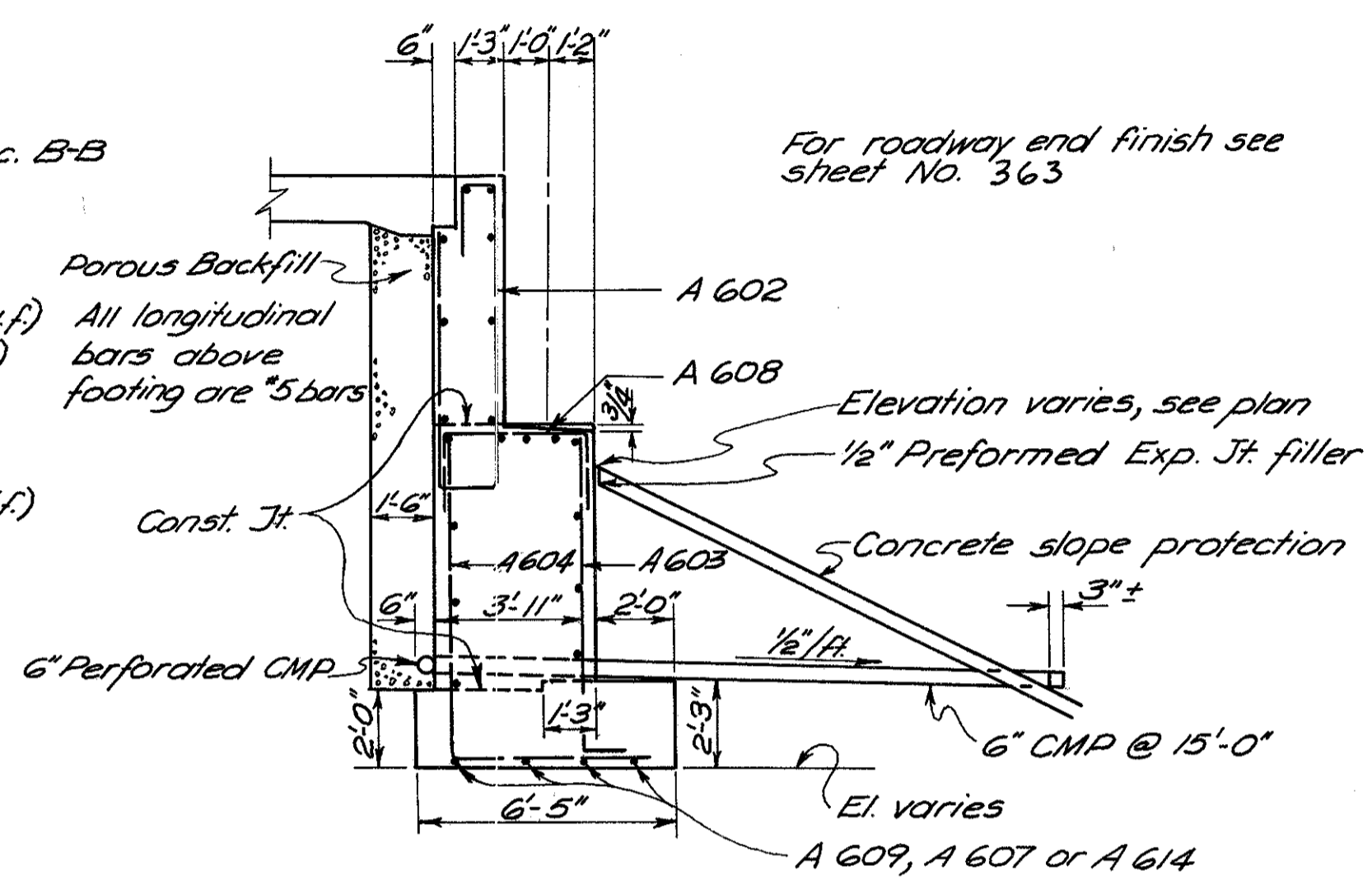
H&E BRIDGE NO. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	A.N.	B. Sch.	J.H.O.	H.A.S.	
			1-21-65	5-20-65	

Type A Aluminum Railing
(for detail see standard
Drawing AR-1-57)

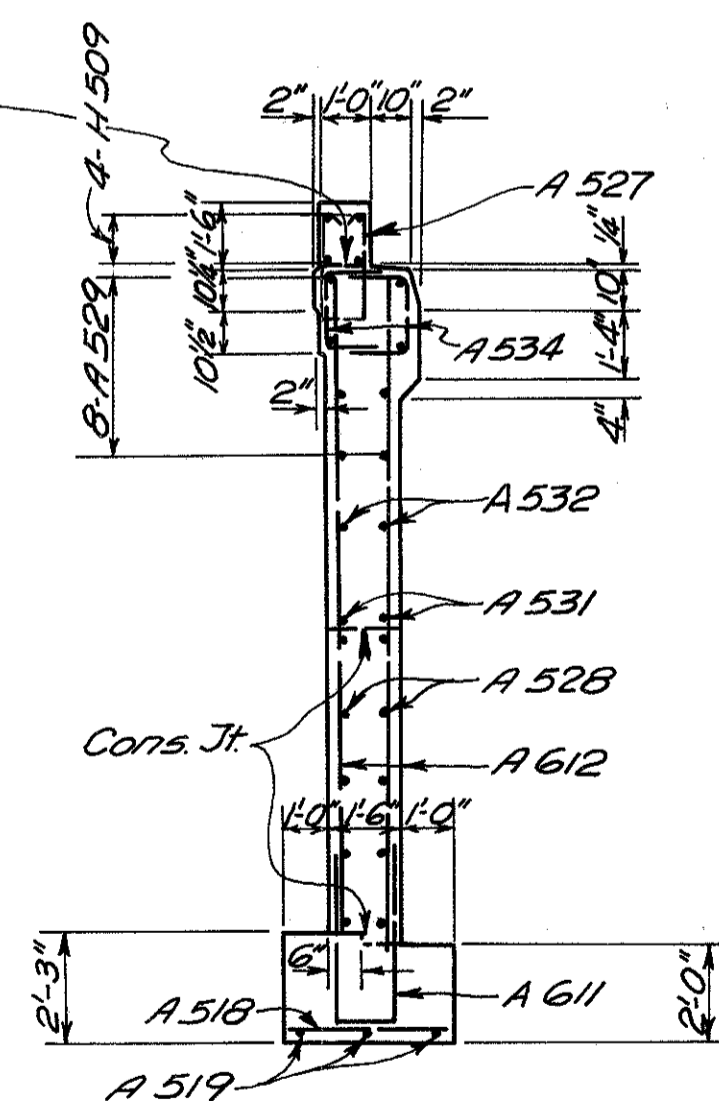


EAST WINGWALL ELEVATION

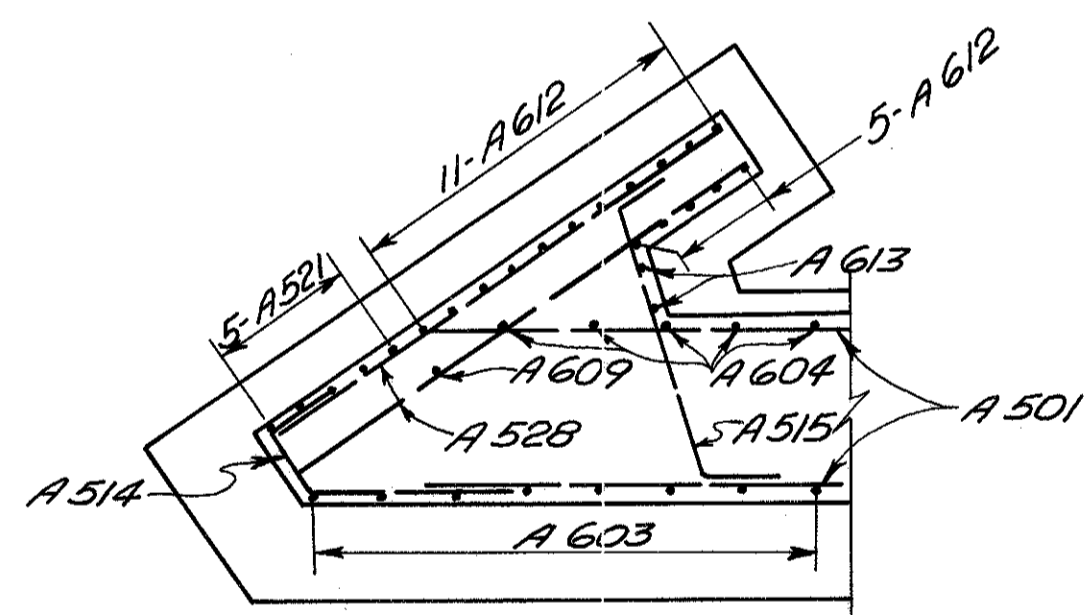


SECTION A-A

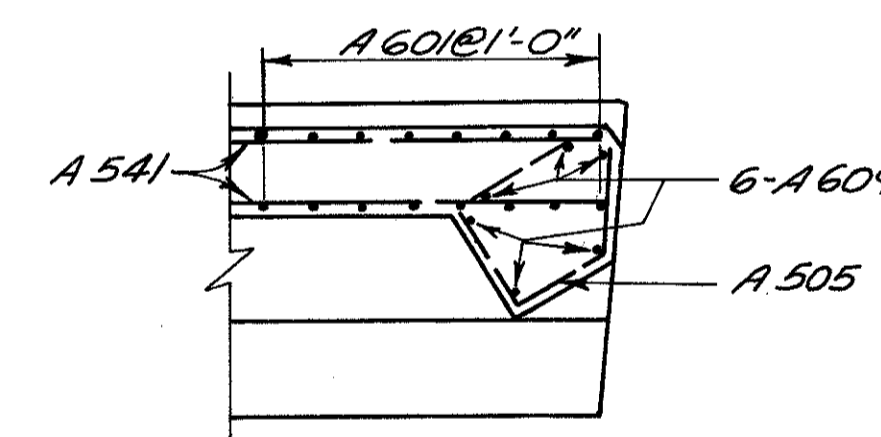
Concrete above this const. Jt.
included with Railing for
payment (Typ)



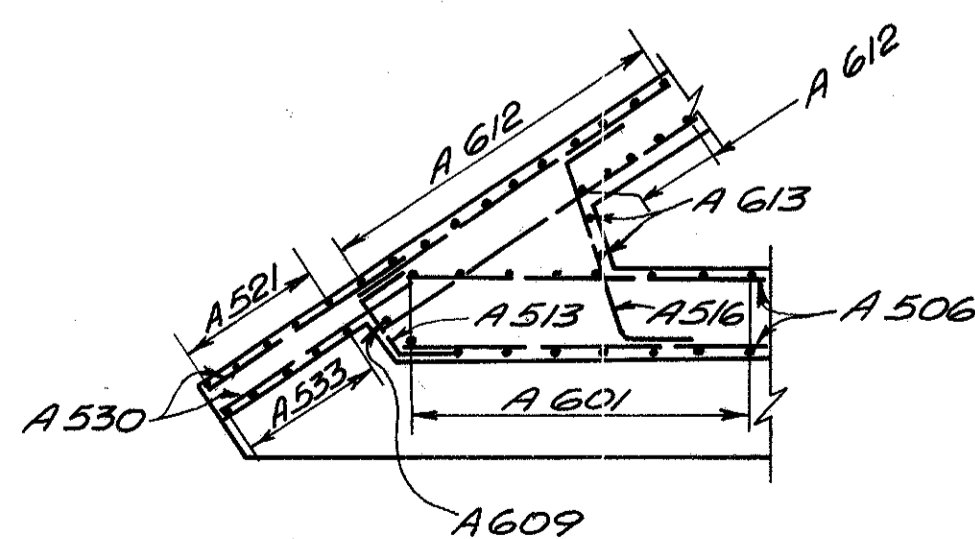
SECTION B-B



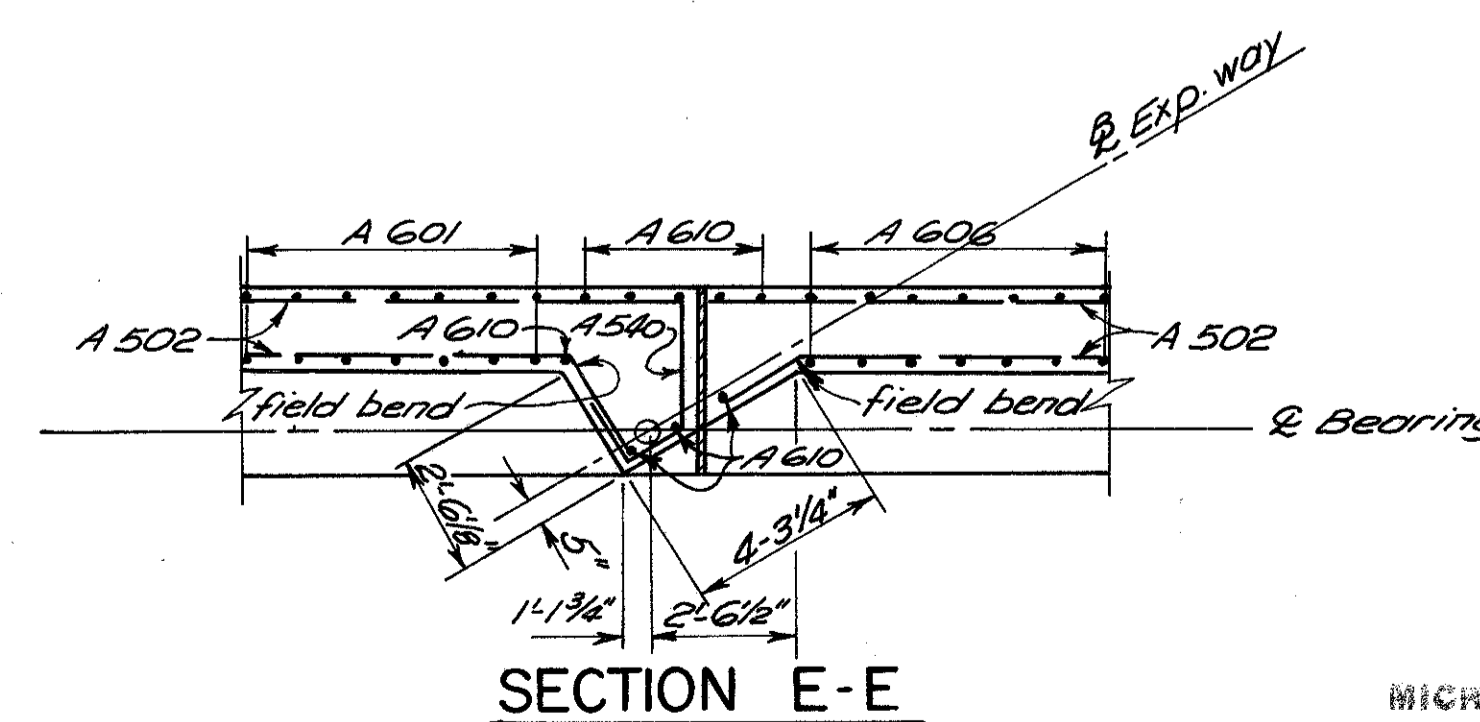
SECTION D-D



SECTION F-F



SECTION C-C



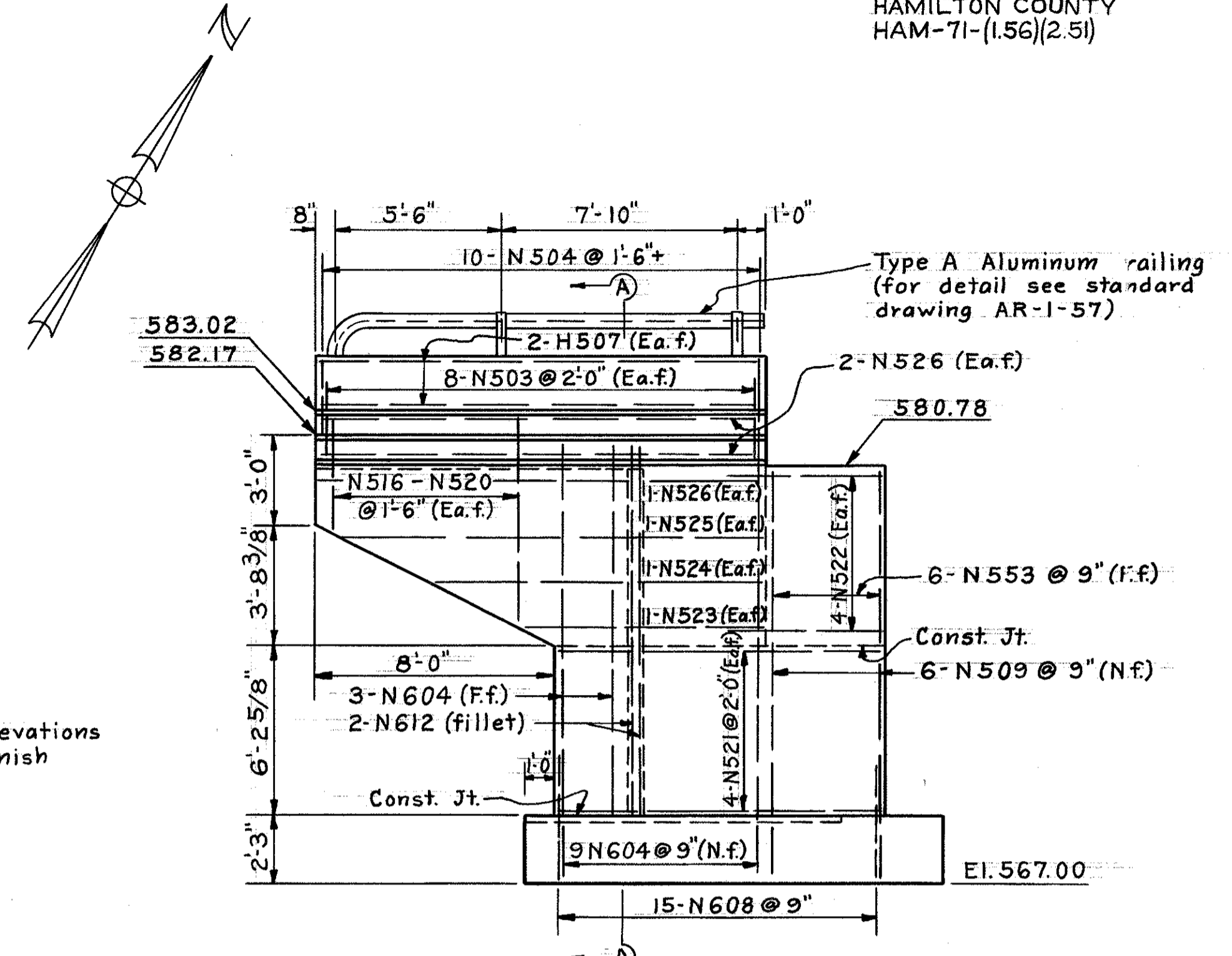
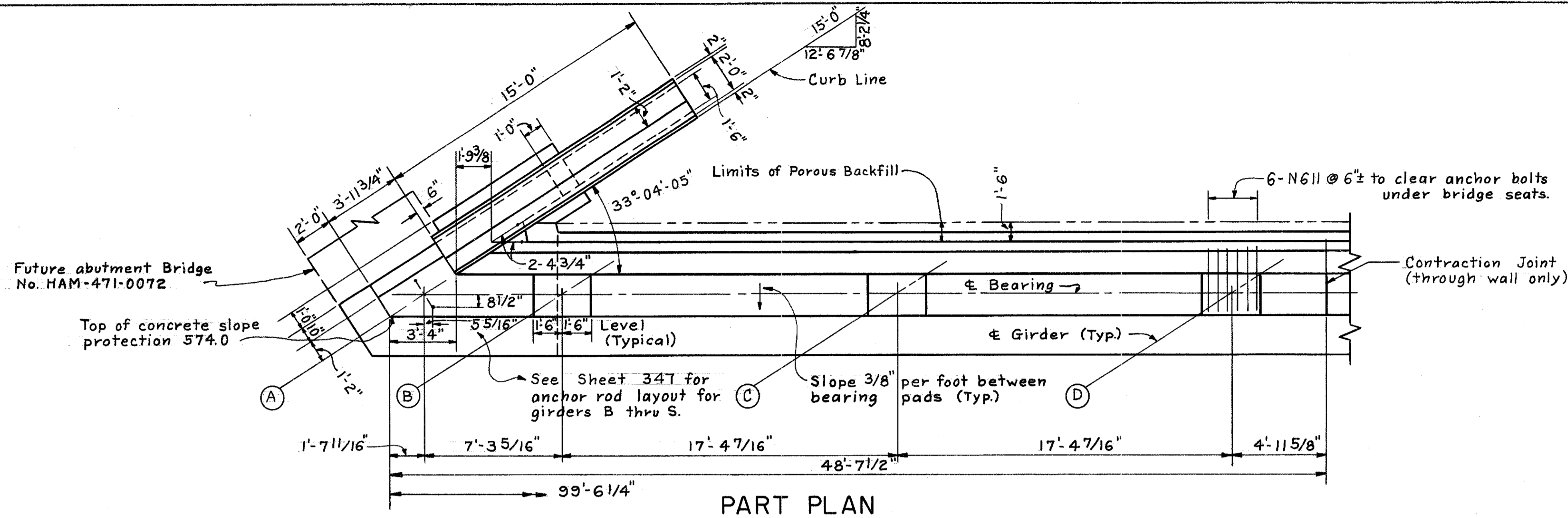
SECTION E-E

MICROFILMED
FEB 20 1985

work this sheet with sheets 345 thro 347

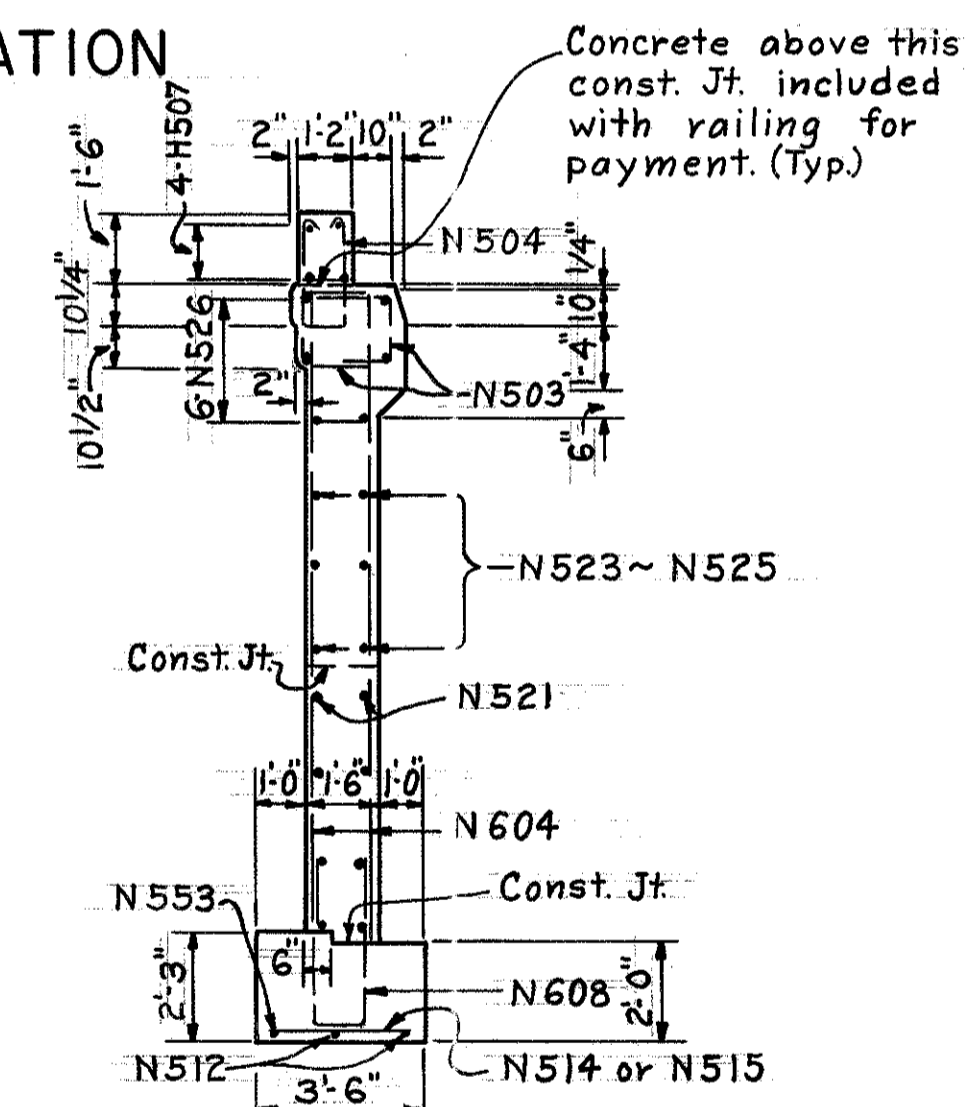
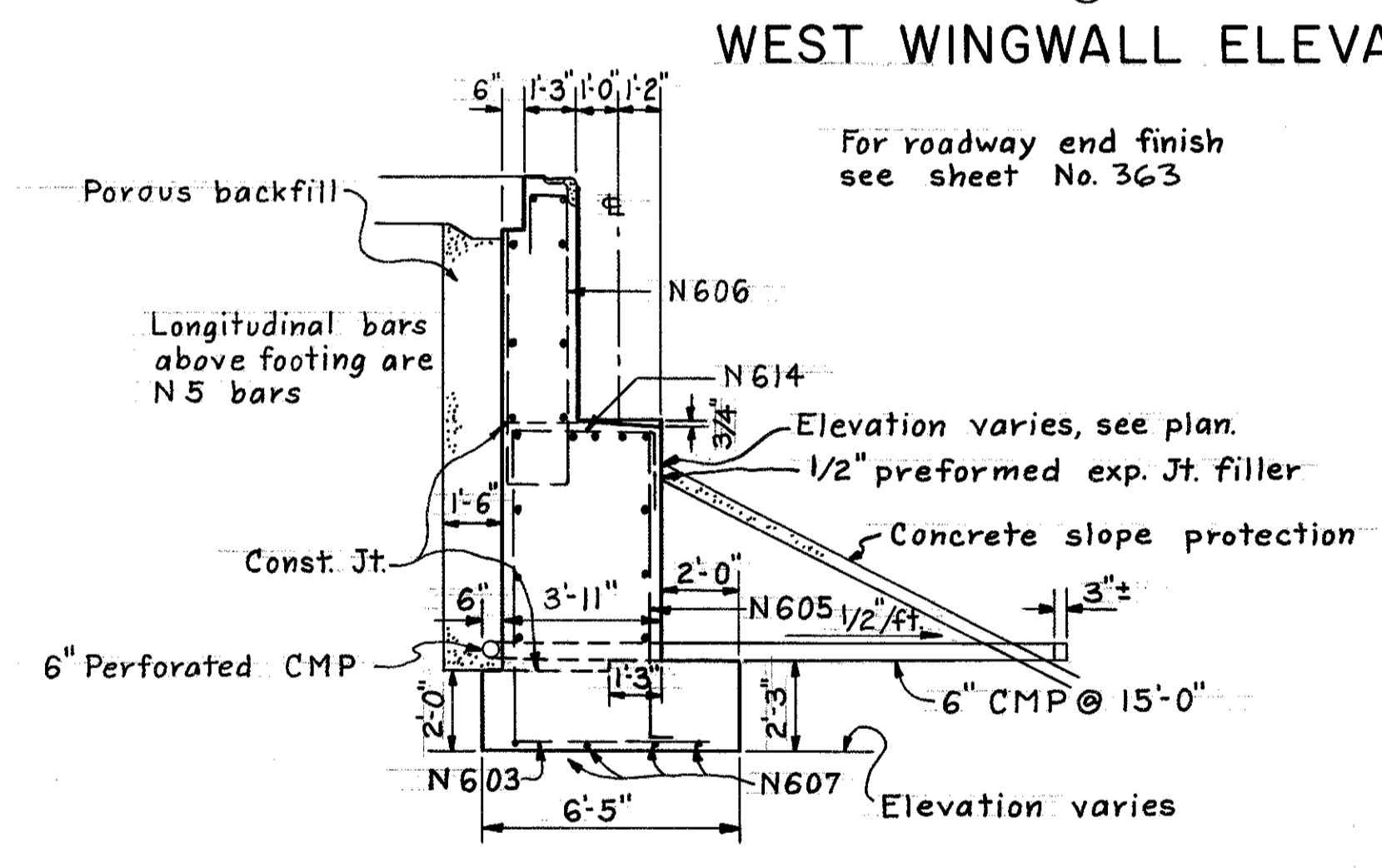
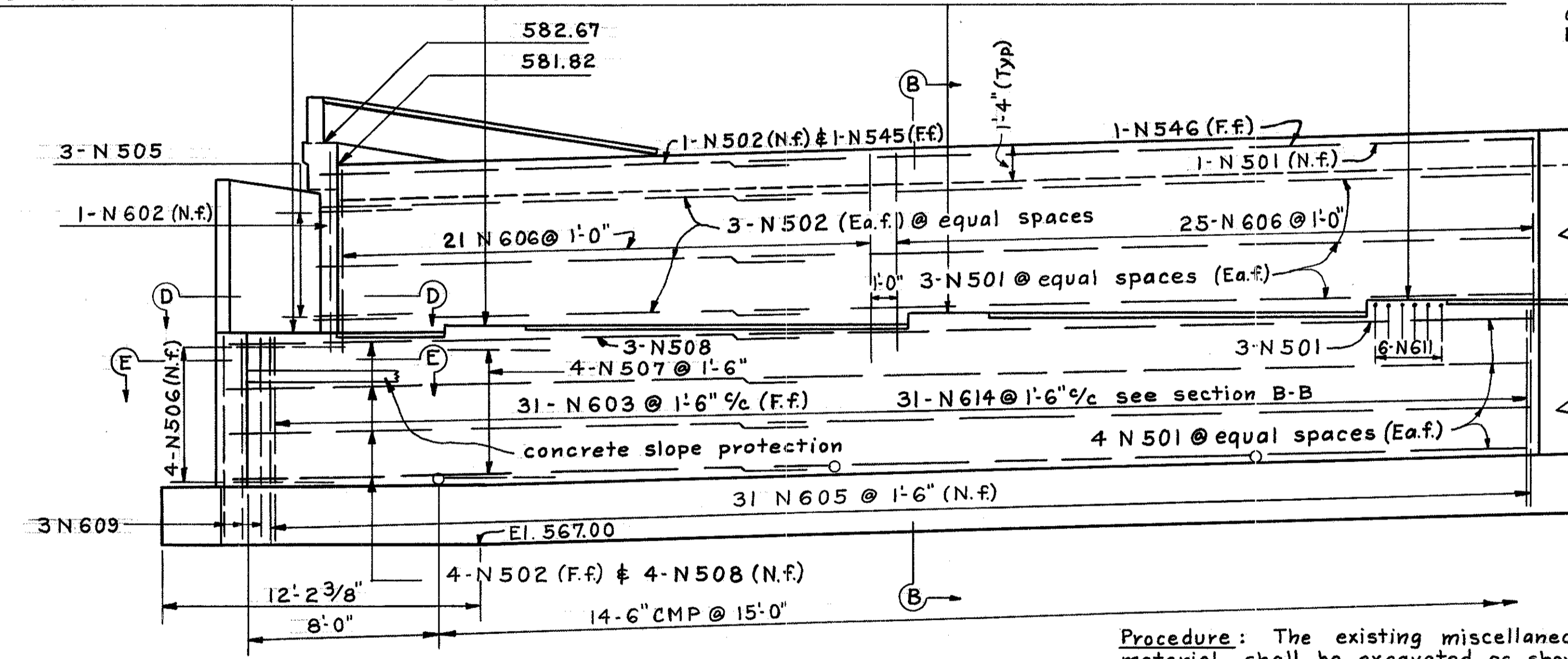
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
SOUTH ABUTMENT					
BRIDGE NO. HAM.-71-0173					
H & E BRIDGE NO. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
A. N.	B. Sch.	J. H. O.	H. A. B.	5-20-65	

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

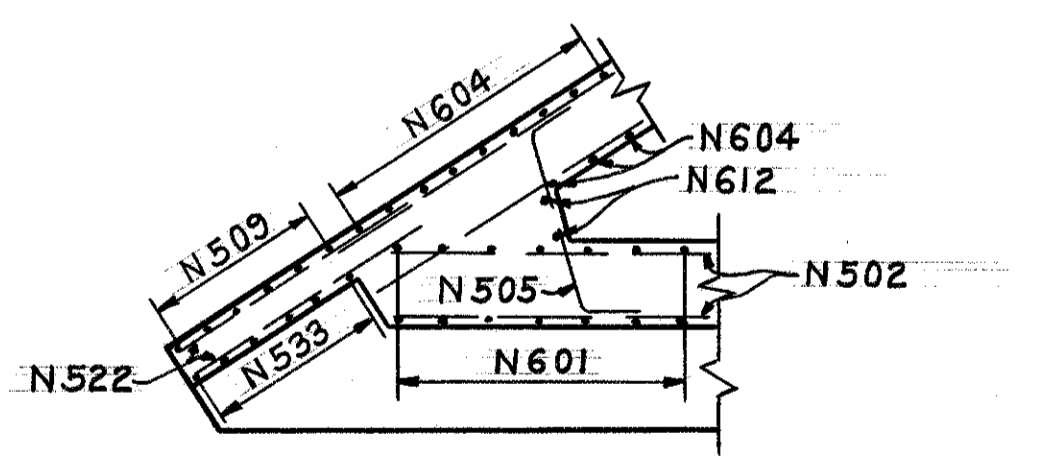


For lighting details See Sh. No. 214.
Bridge Seat Elevations: 575.47 575.69 576.22 576.74

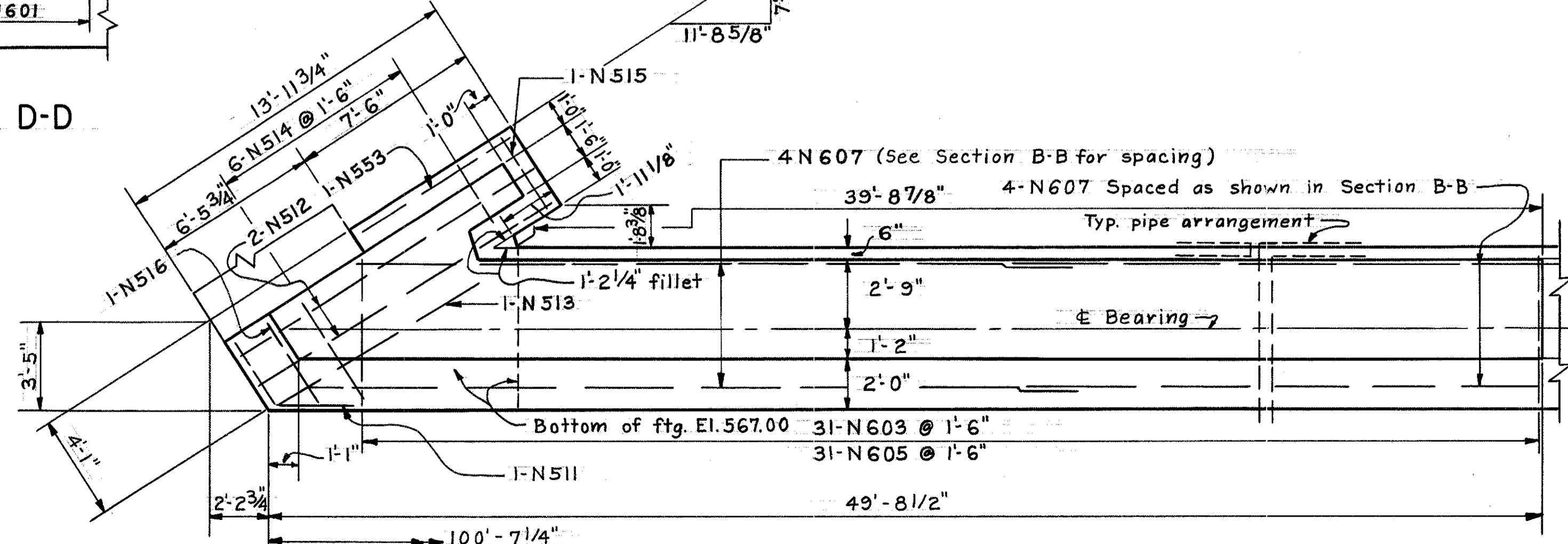
Note: For top of backwall elevations along backwall see End Finish Details Sh. No. 363



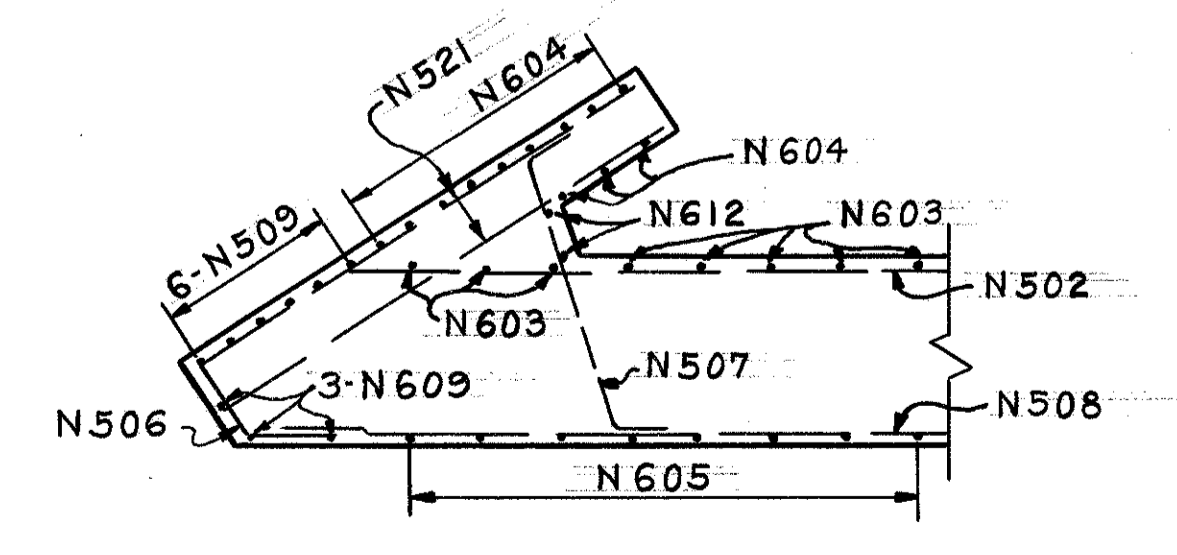
Procedure: The existing miscellaneous fill material shall be excavated as shown on sheet No. 342, then replaced and compacted with embankment material. The embankment shall be placed and compacted up to the finished spill-thru slope and to the level of the subgrade for a distance of 200 feet back of the abutment. Excavation for the abutment shall not be done until a minimum period of 60 days after completion of the embankment.



SECTION D-D



PART FOOTING PLAN



SECTION E-E

MICROFILMED
FEB 20 1985

Work this sheet with sheets 350 & 351

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

NORTH ABUTMENT
BRIDGE No. HAM.-71-0173

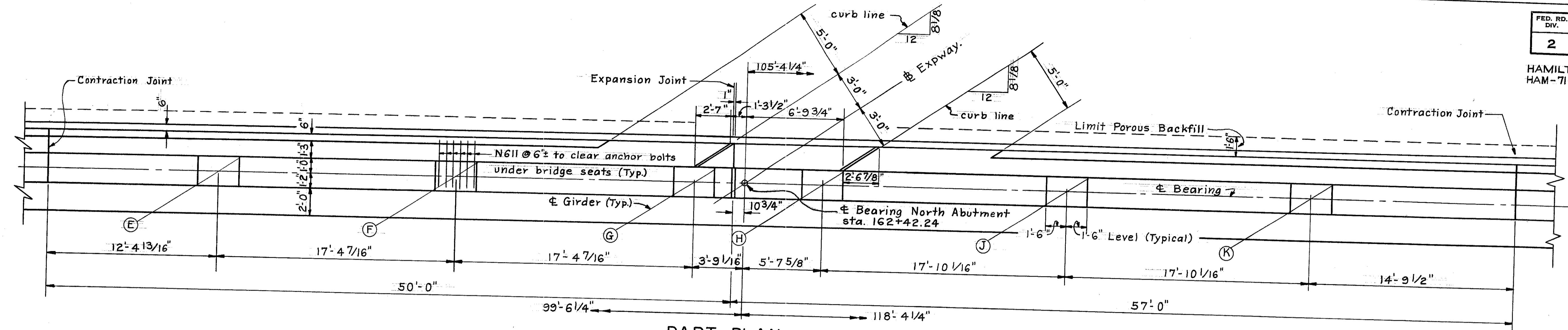
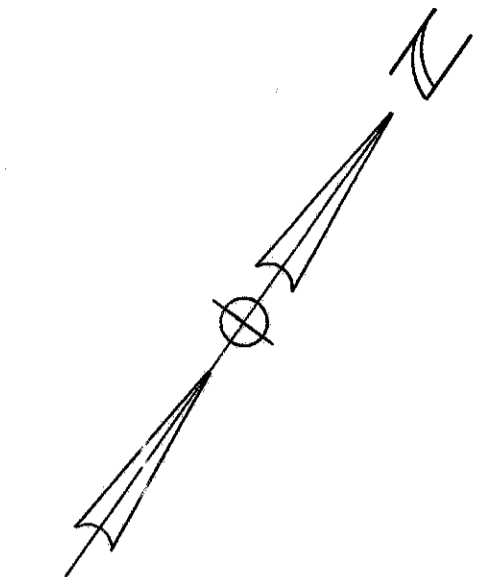
H & E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVIEWED
	AN	RS	JHO	7/1/65	

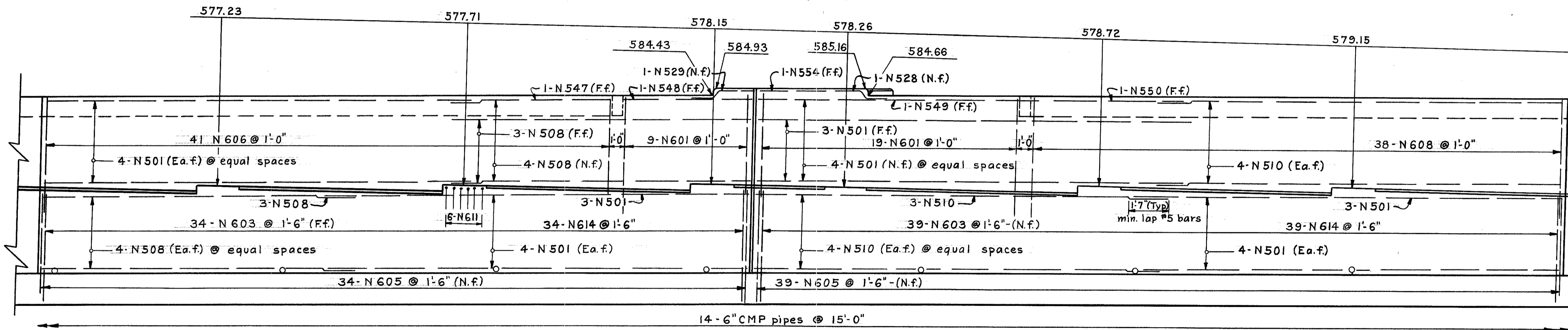
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

350
492

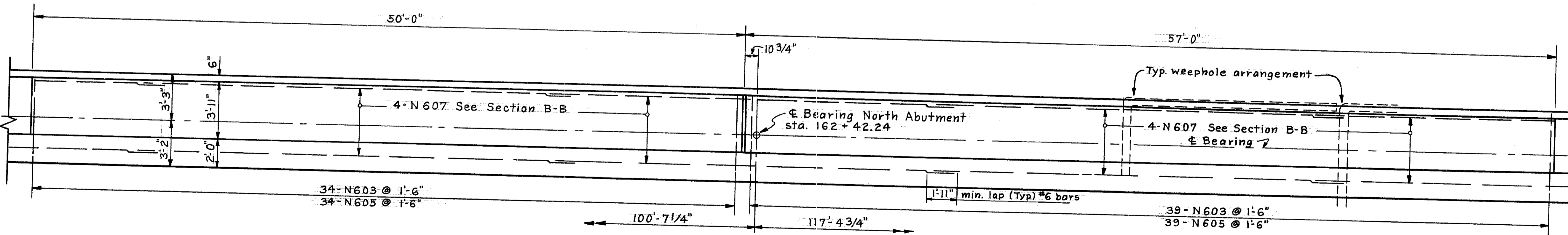
HAMILTON COUNTY
HAM-71-(156)(2.51)



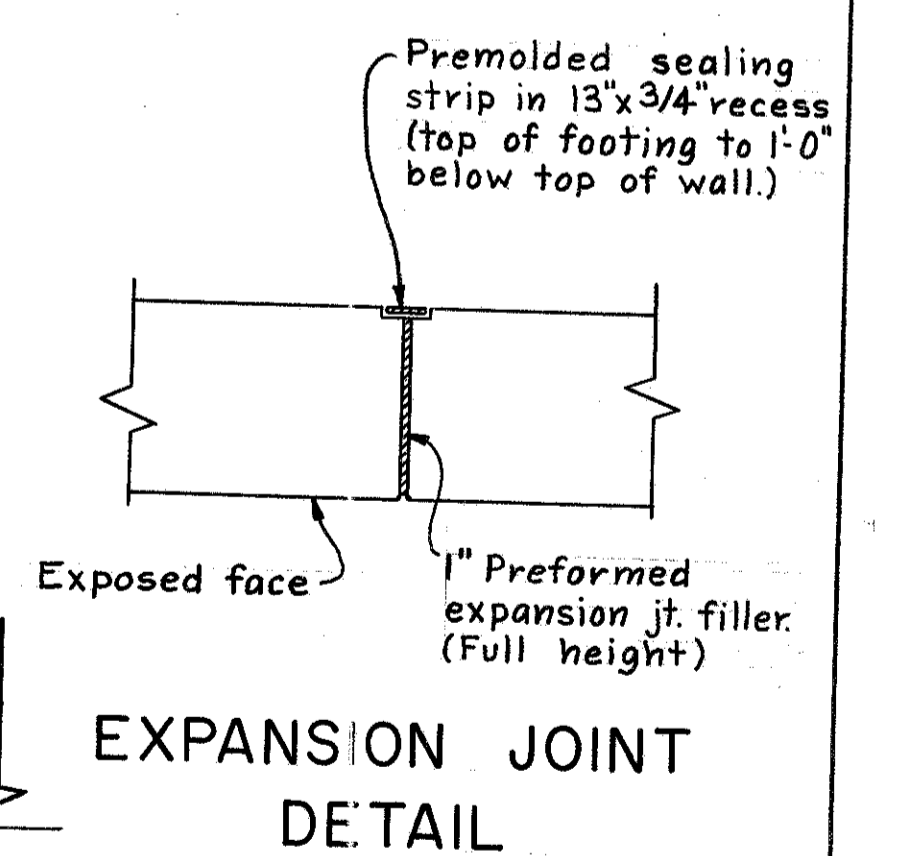
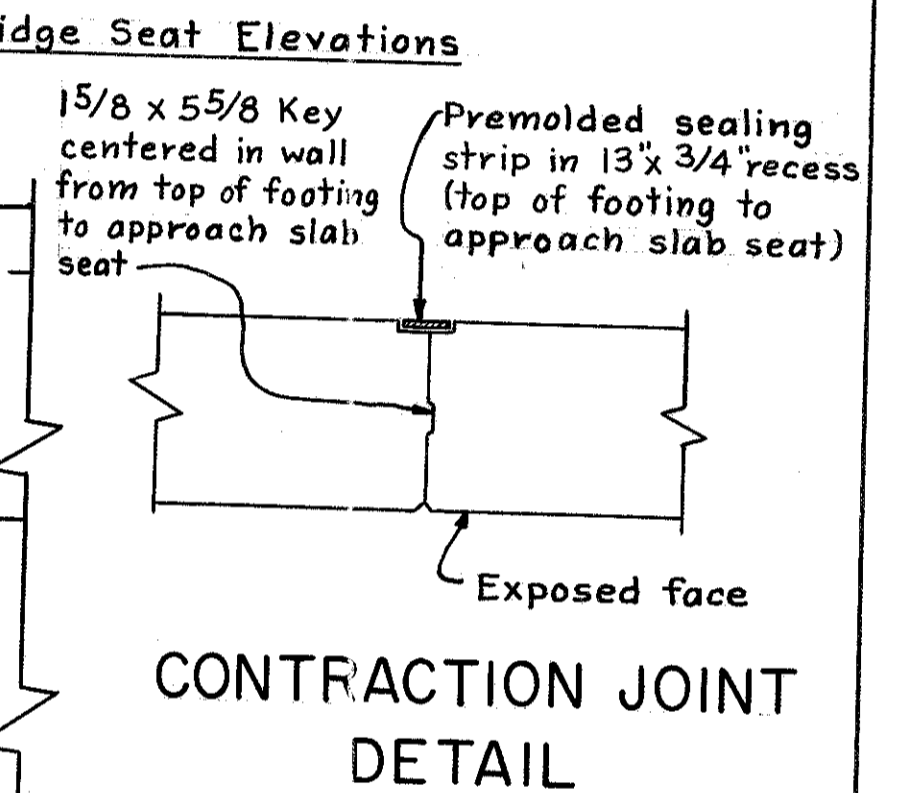
PART PLAN



PART ELEVATION



PART FOOTING PLAN



NOTES:
N.f. is near face; F.f. is far face; Ea.f. is each face.
Provide 3" clearance to reinforcing steel in footing and 2" clearance in wall minimum.
Concrete above bridge seat const. jt. shall be placed after structural steel is in place.
All concrete Class "E"
Top of backwall elevations shown are at the near face of backwall.
Special care shall be taken in placing steel in the bridge seat, so that it will not interfere with the drilling of anchor rod holes.

MICROFILMED
FEB 20 1985

Work this sheet with sheets 349 & 351

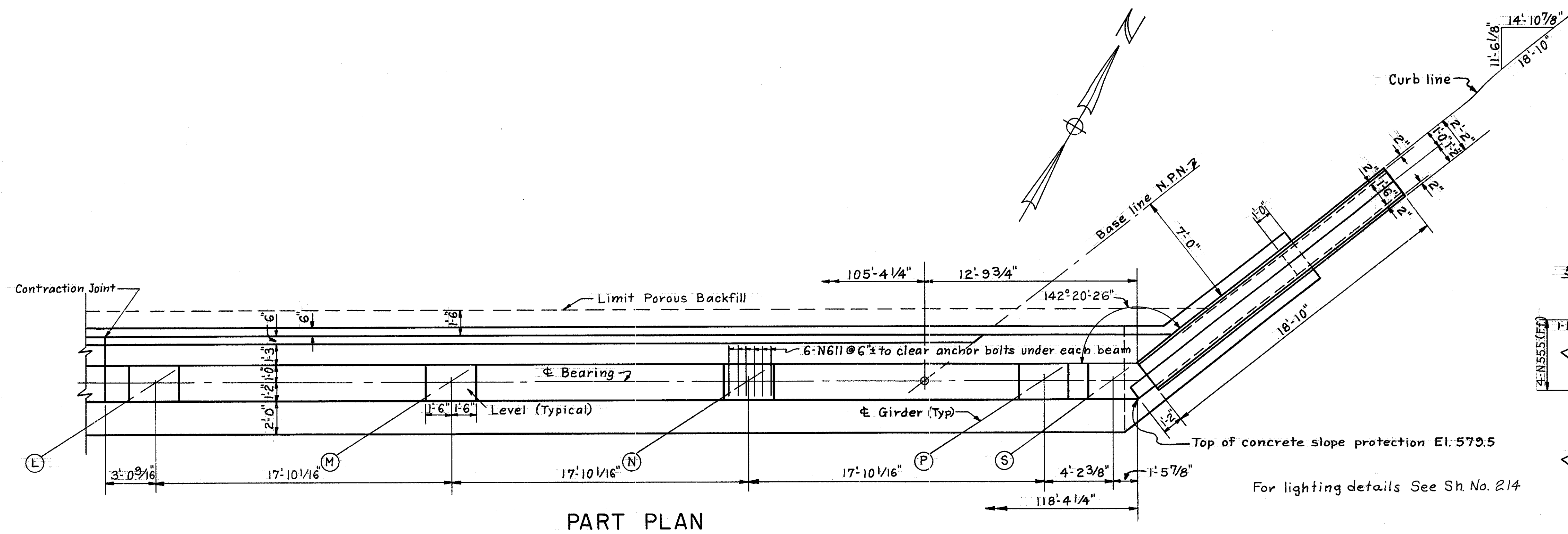
HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

NORTH ABUTMENT
BRIDGE No. HAM.-71-0173

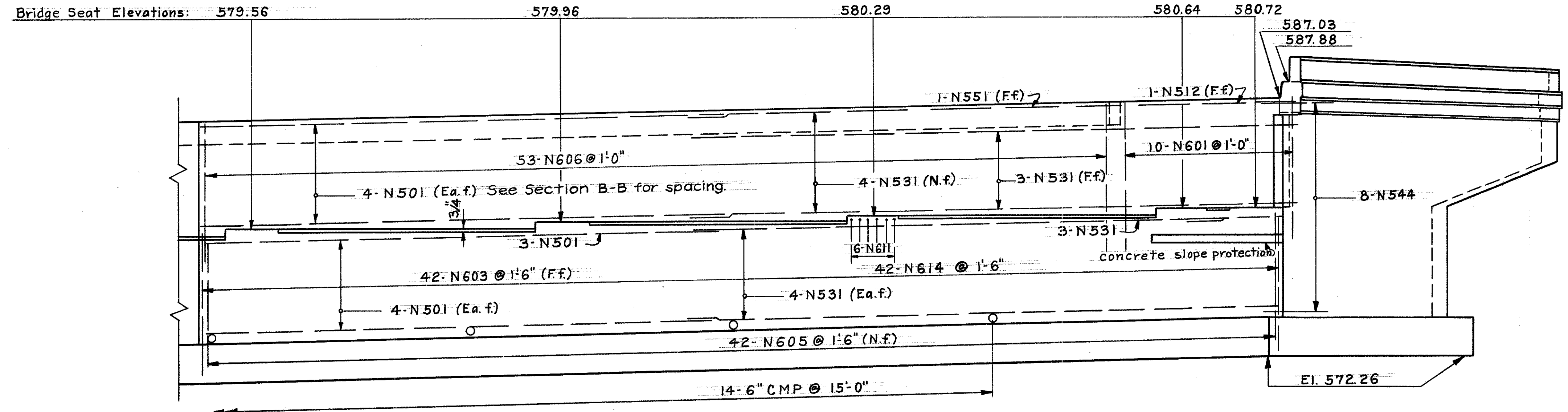
H & E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
AN	RS	JHC	11/1/65	5-22-65	

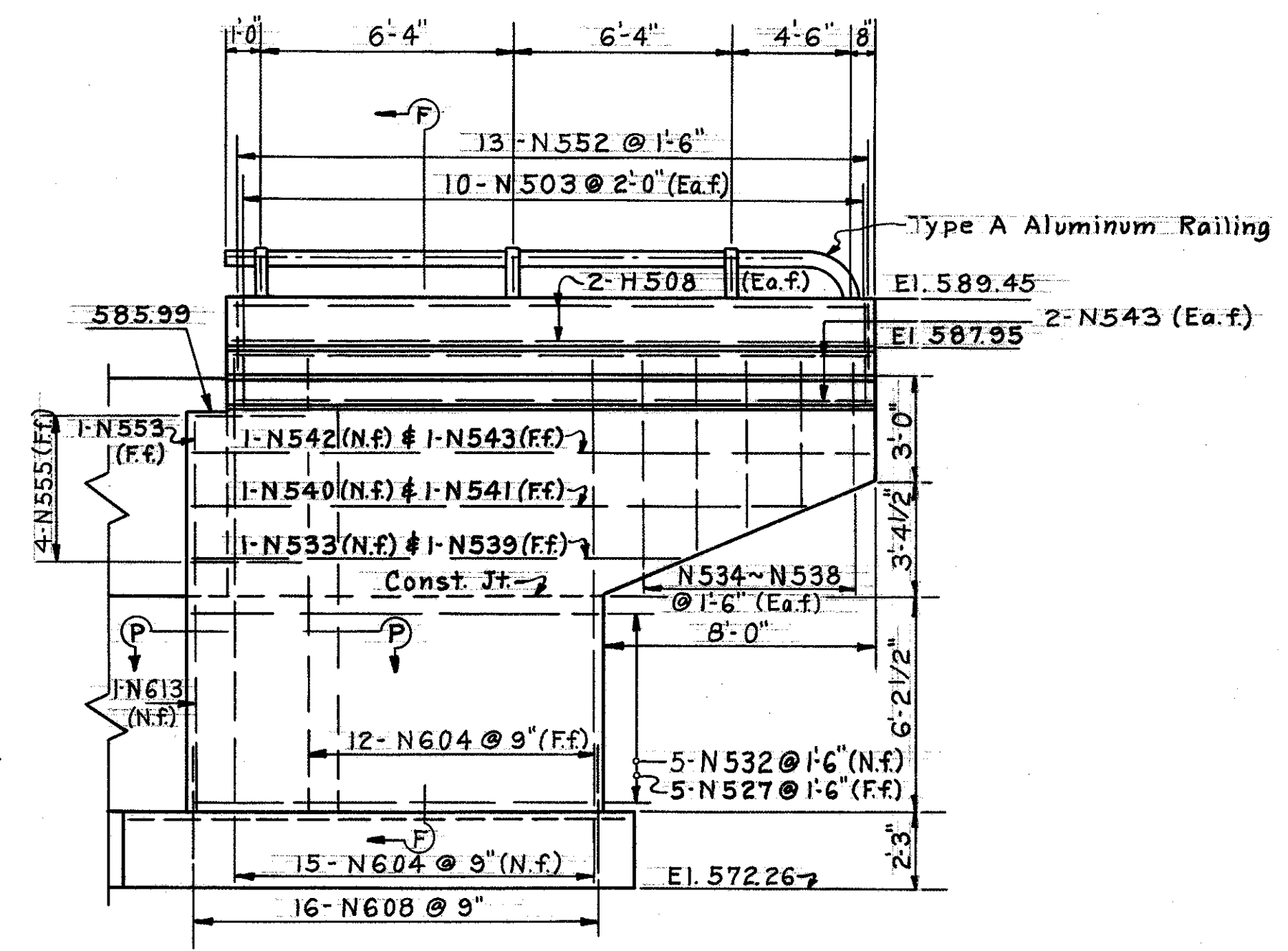
HAMILTON COUNTY
HAM-71-(1.5C)(2.5I)



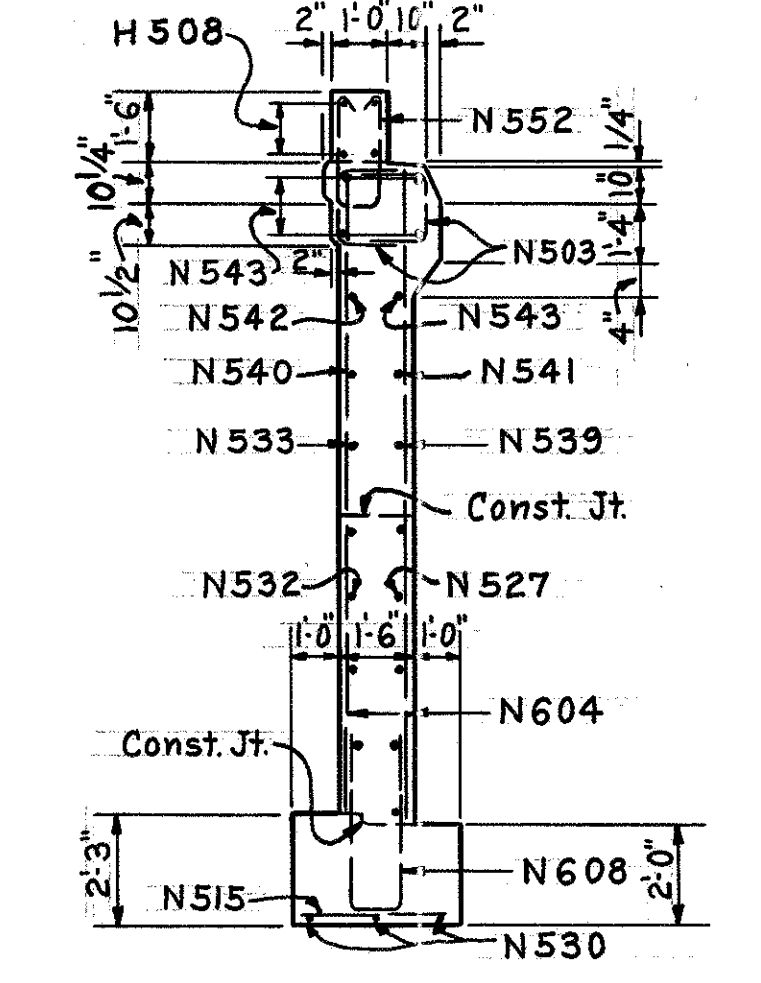
PART PLAN



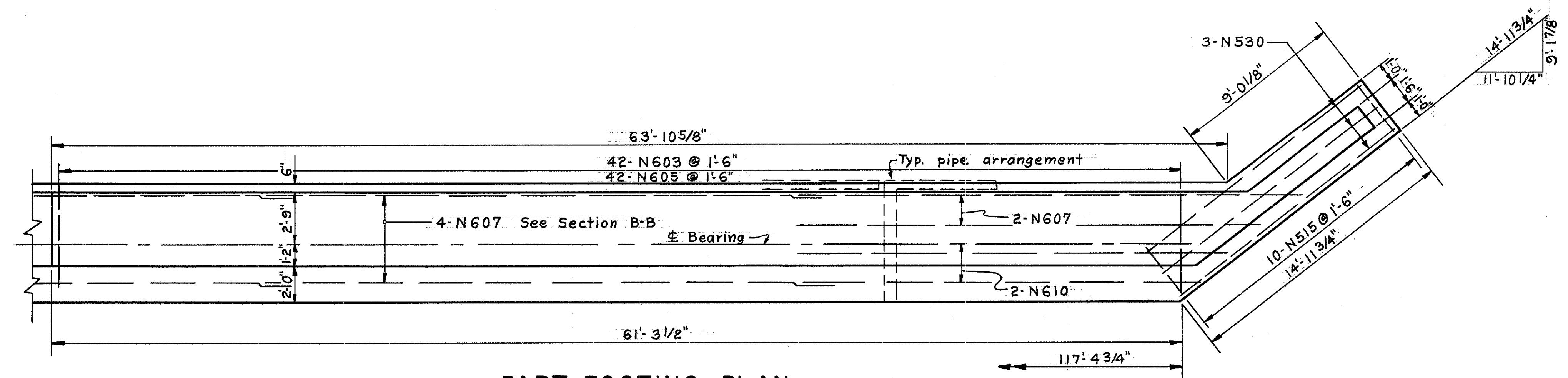
PART ELEVATION



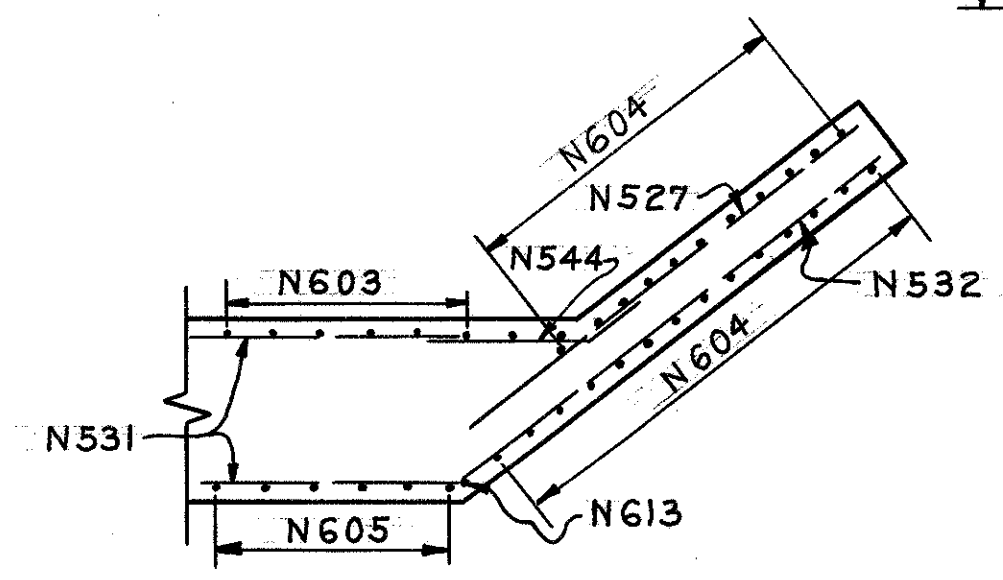
EAST WINGWALL ELEVATION



SECTION F-F



PART FOOTING PLAN



SECTION P-P

Work this sheet with sheets 349 & 350

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

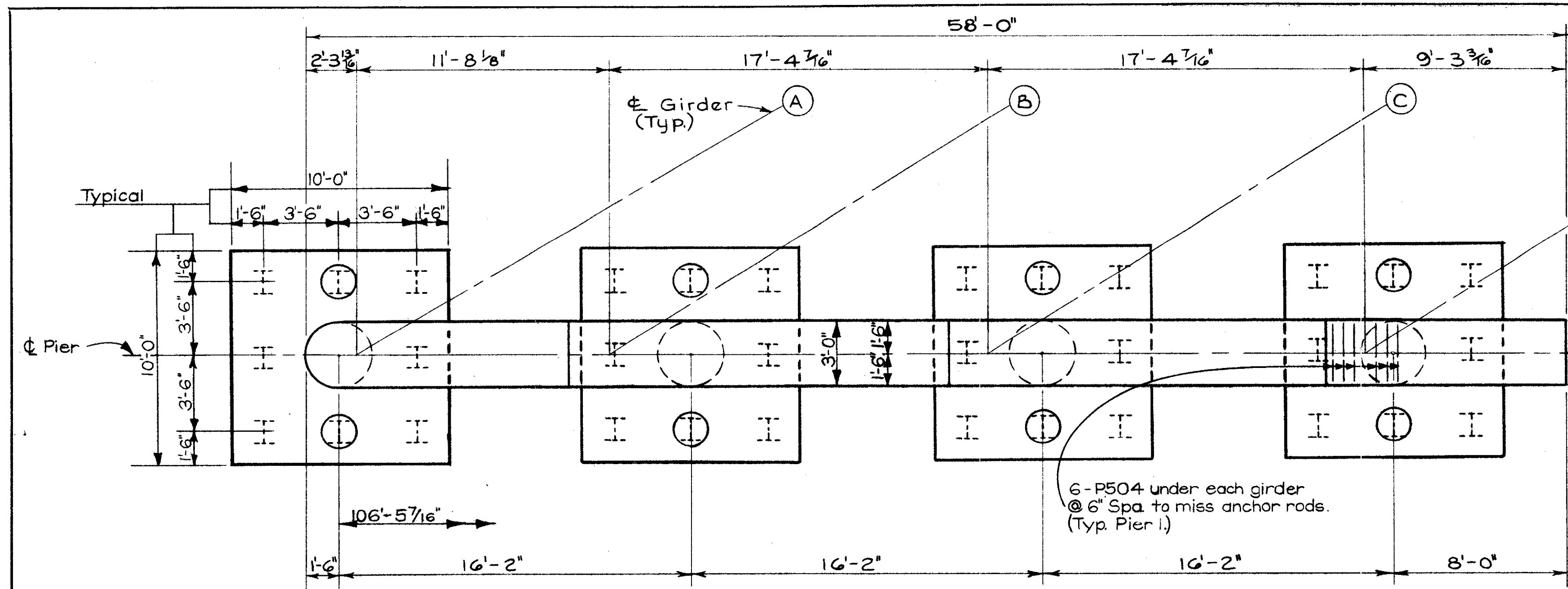
NORTH ABUTMENT
BRIDGE No. HAM-71-0173

H&E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	A.N.	RS	JHO	5-20-65	

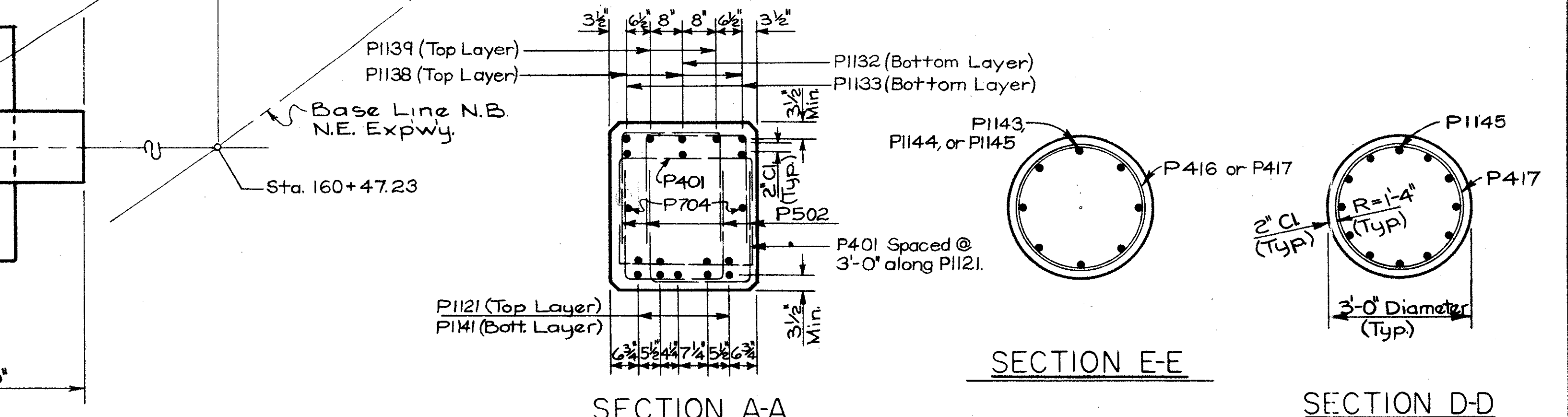
RECORDED
FEB 20 1965

HAMILTON COUNTY
HAM-71- (1.56) (2.51)



PLAN

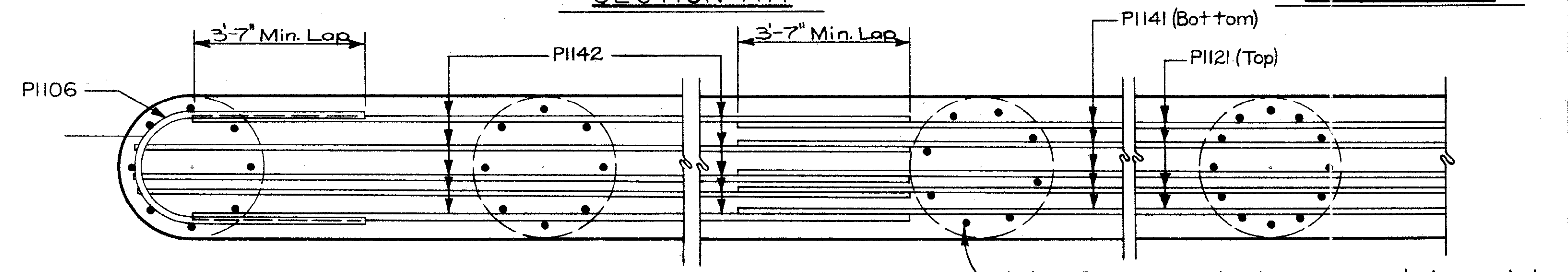
⊙ Indicates battered pile.



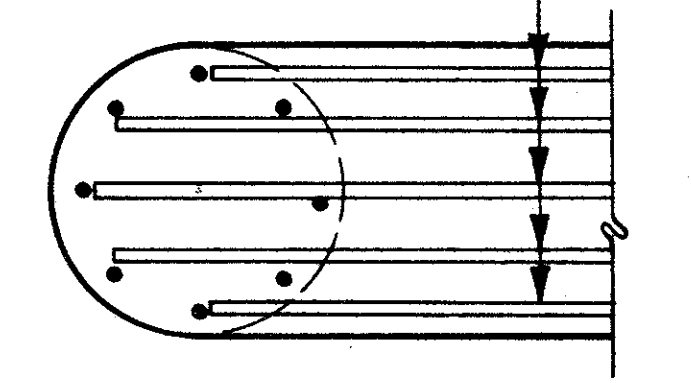
SECTION A-A

SECTION E-E

SECTION D-D



SECTION B-B



SECTION C-C

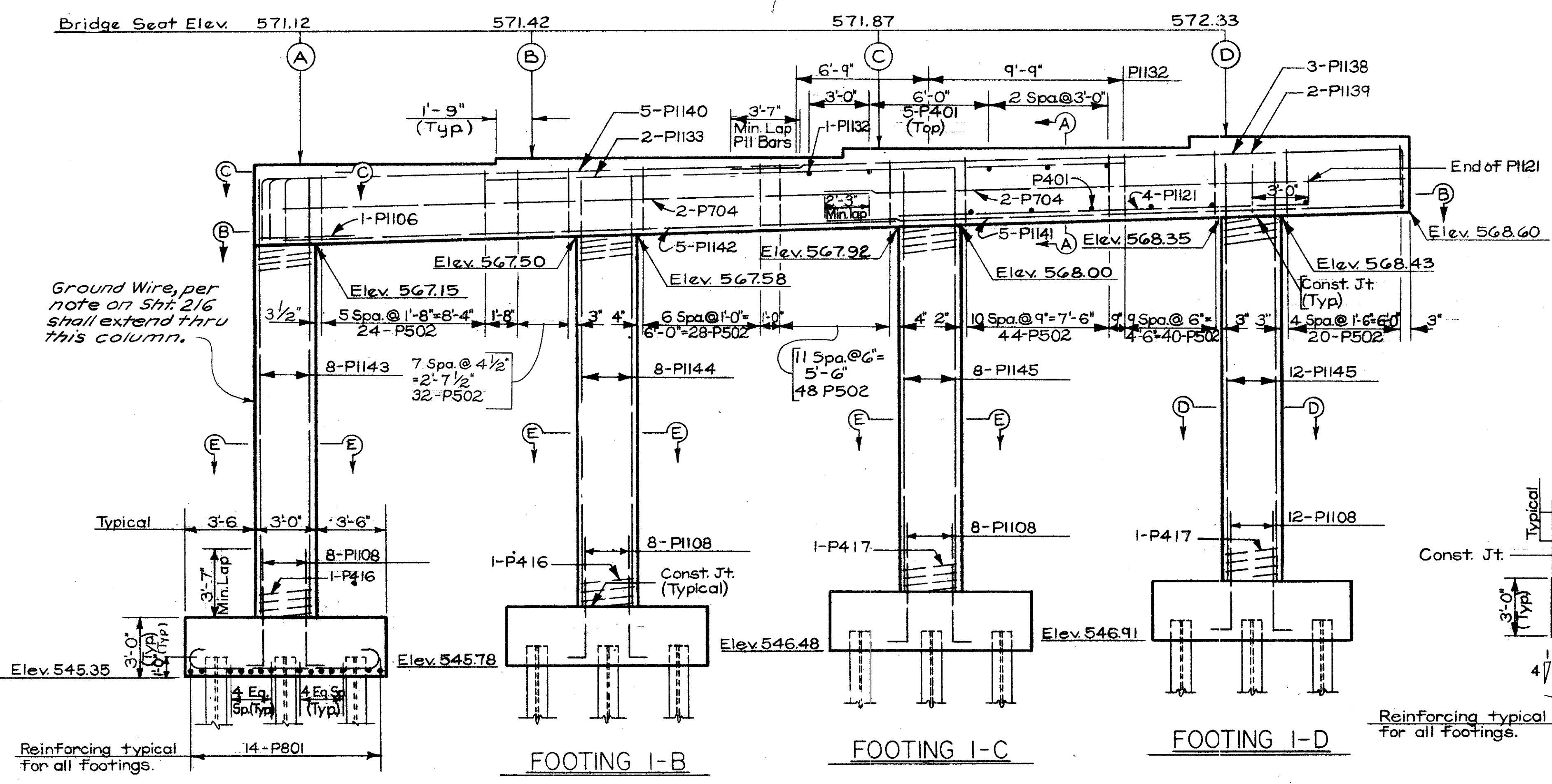
Note: Bars are to be spaced to match bottom cap reinforcing as dimensioned in Section A-A.

NOTES: (Applies to all piers)

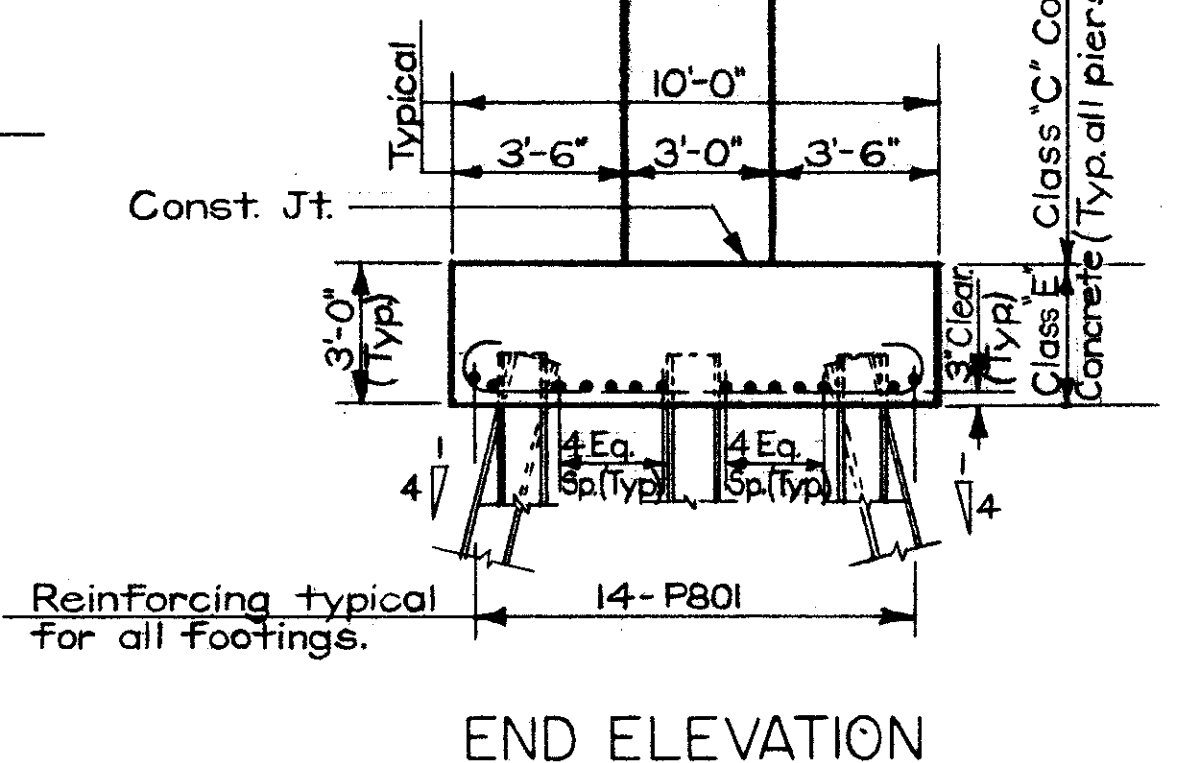
Special care shall be taken in placing steel in the pier cap so that it will not interfere with the drilling of anchor rod holes. Provide 3" clearance to reinforcing steel in footing and 2" clearance to reinforcing steel in cap and column.

All piles shall be 10" BP 42.

For location of 2" galv. metal conduit in Pier See Sh. No. 214



ELEVATION



END ELEVATION

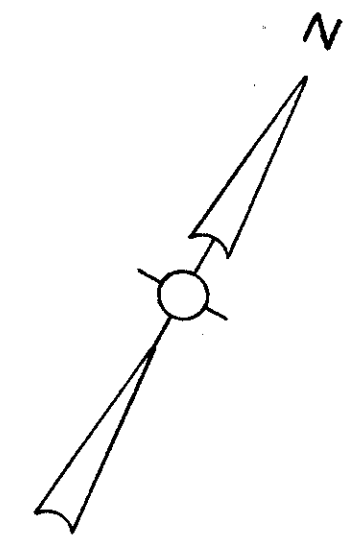
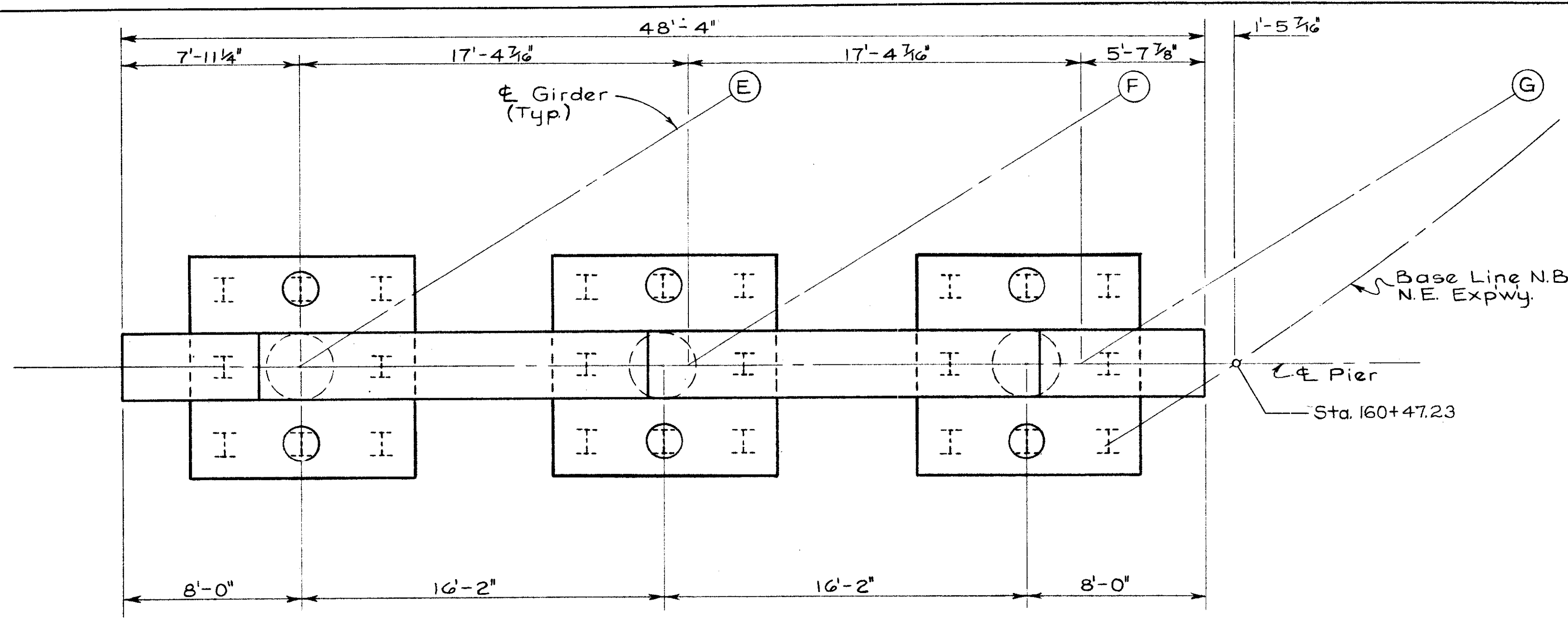
Reinforcing typical for all Footings.

Reinforcing typical for all Footings.

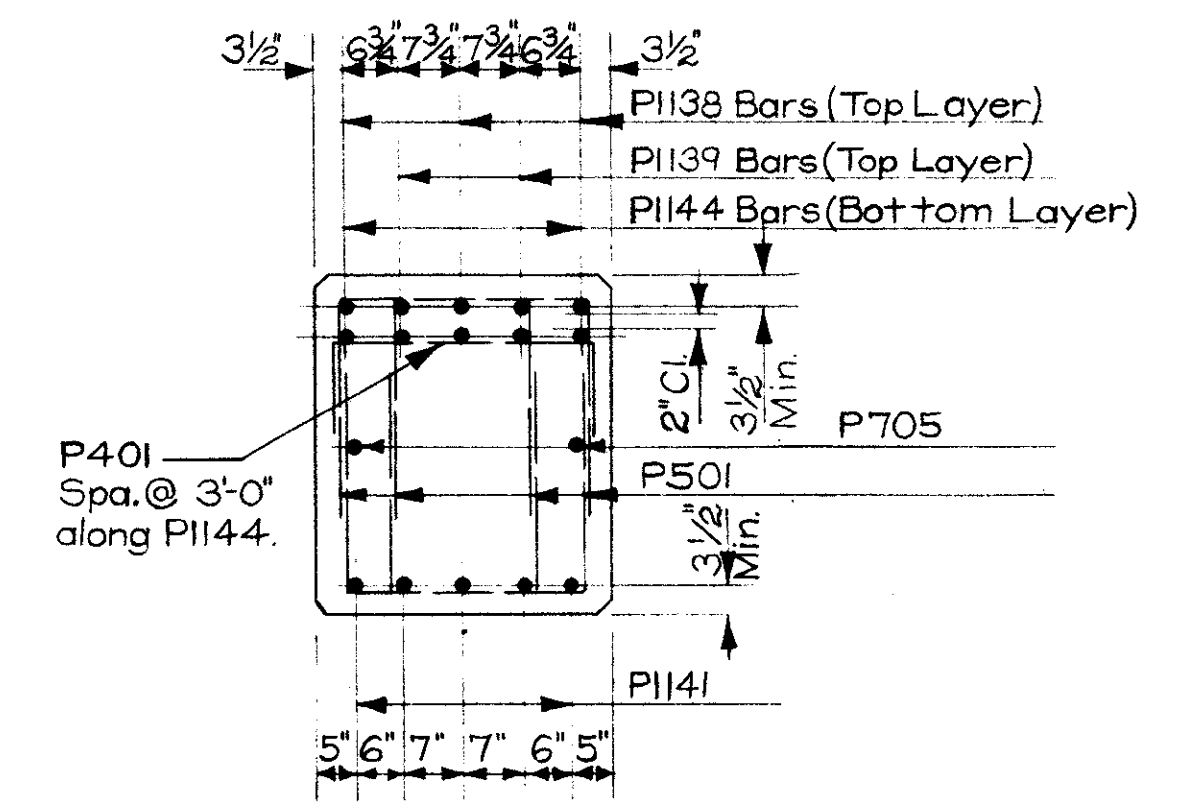
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
PIER NO. 1				
BRIDGE No. HAM-71-0173				
H&E BRIDGE No. 8				
DESIGNED	DRAWN	CHECKED	REVIEWED DATE	REVISION
JDC	JH	JH	5-20-65	

MICROFILMED
FEB. 20 1985

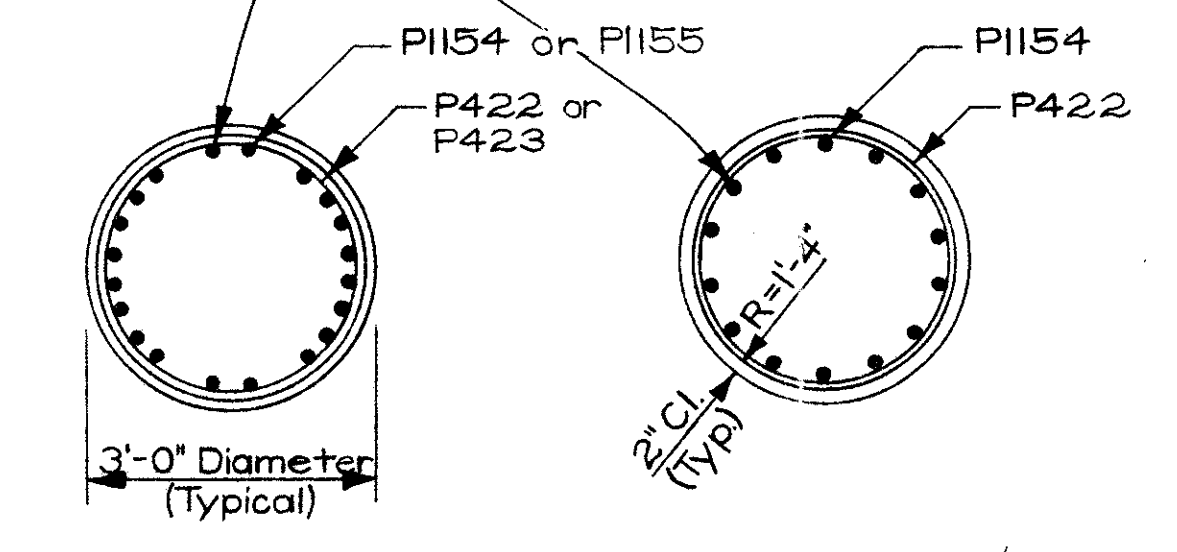
HAMILTON COUNTY
HAM-71-(1.56) (2.51)
Note: Bars are to be spaced to match bottom cap reinforcing as dimensioned in Section A-A.



PLAN

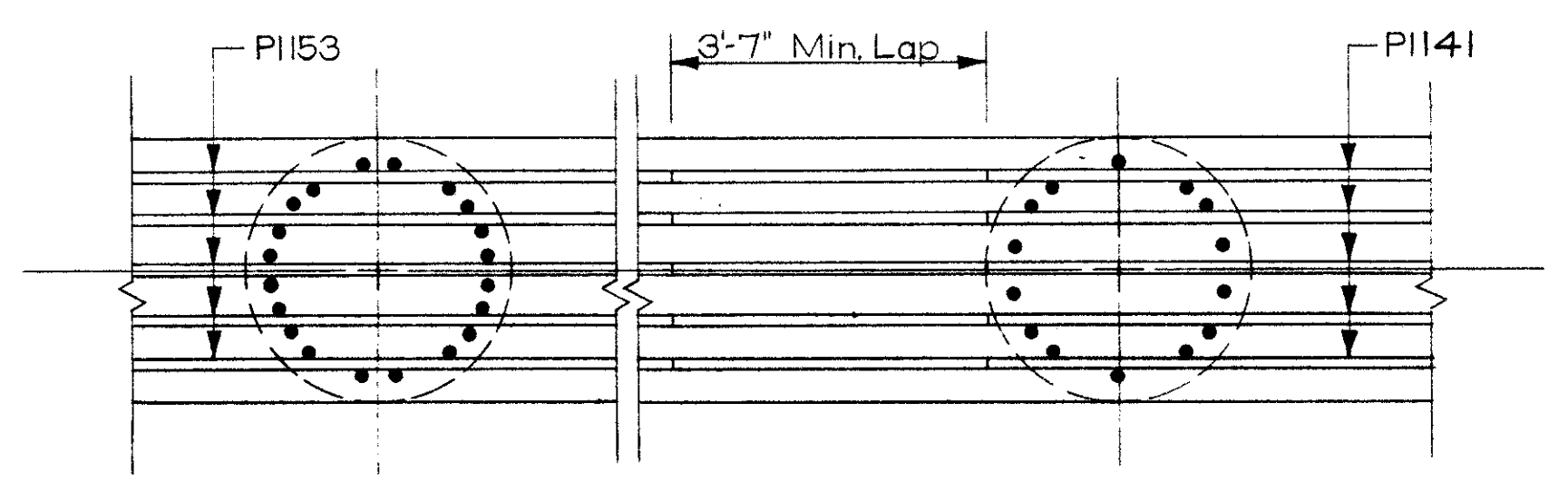


SECTION A-A

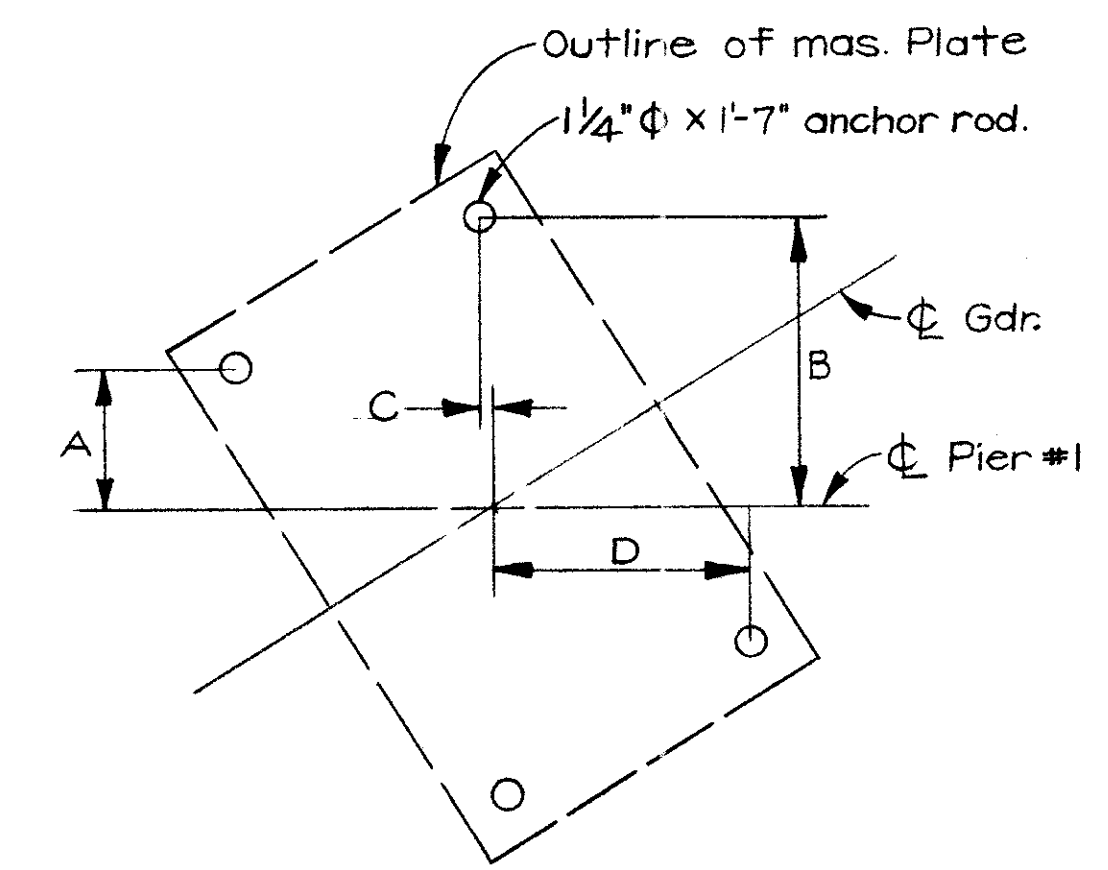


SECTION C-C

SECTION D-D

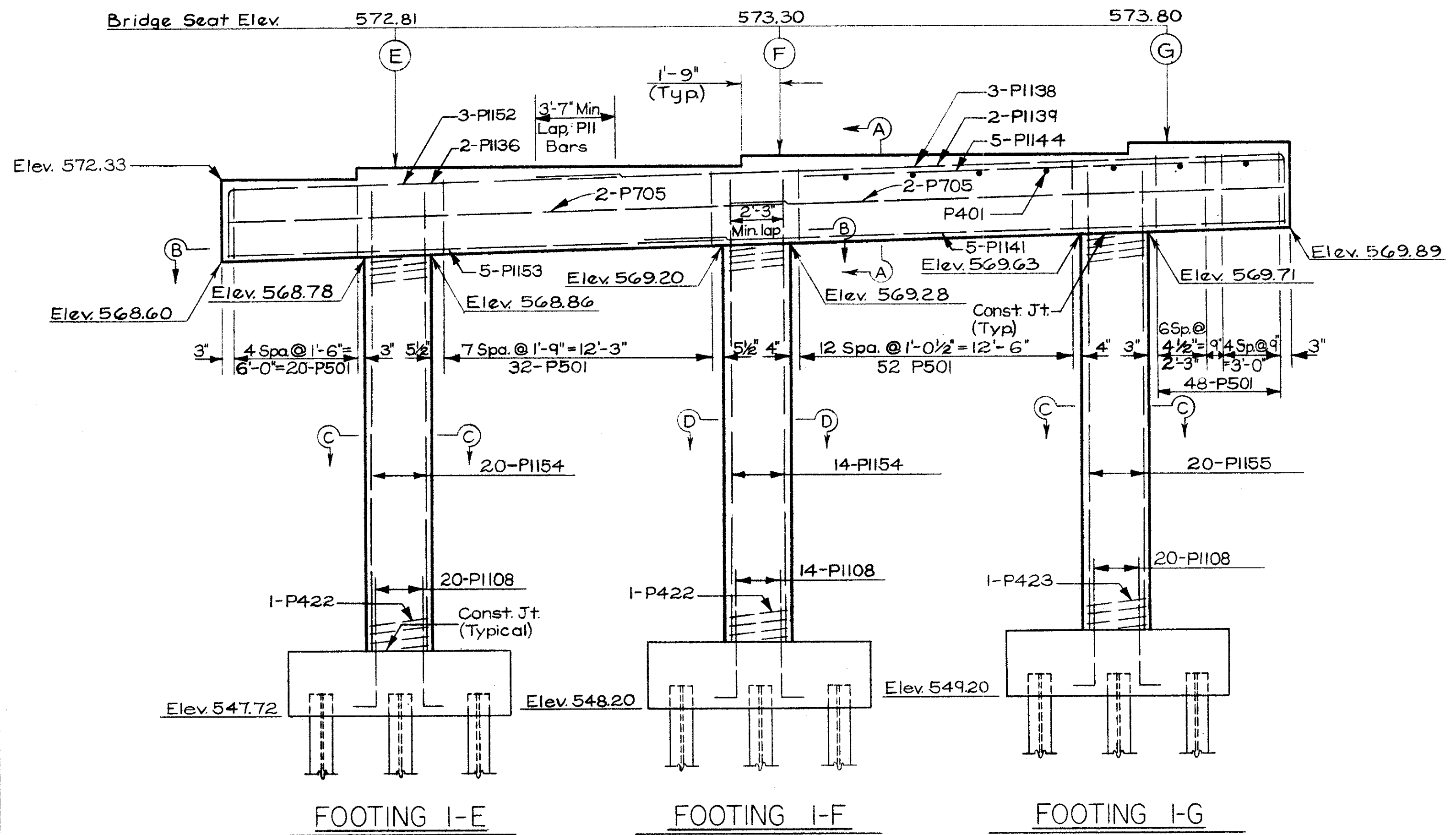


SECTION B-B

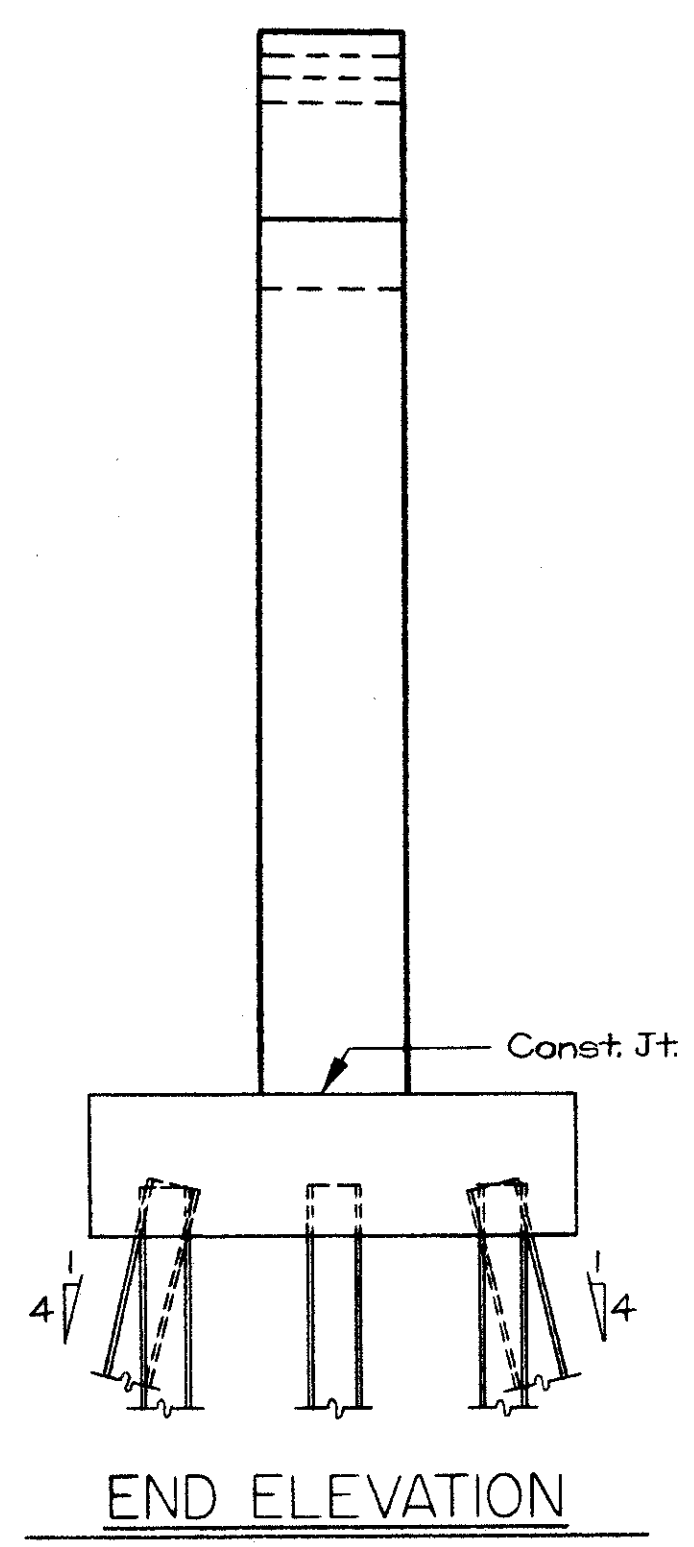


ANCHOR ROD LAYOUT

Girder	A	B	C	D
A	6"	1'-0 1/8"	1/8"	10 1/2"
B thru P	5 1/16"	1'-0 1/16"	1/2"	10 1/16"
R	5 3/8"	1'-0 1/16"	7/8"	10 13/16"
S	5"	1'-0"	1 5/16"	11"



ELEVATION



END ELEVATION

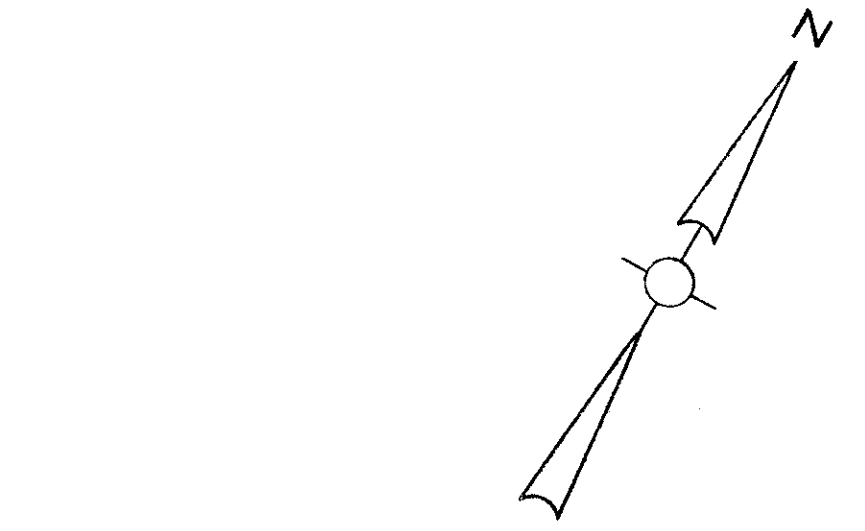
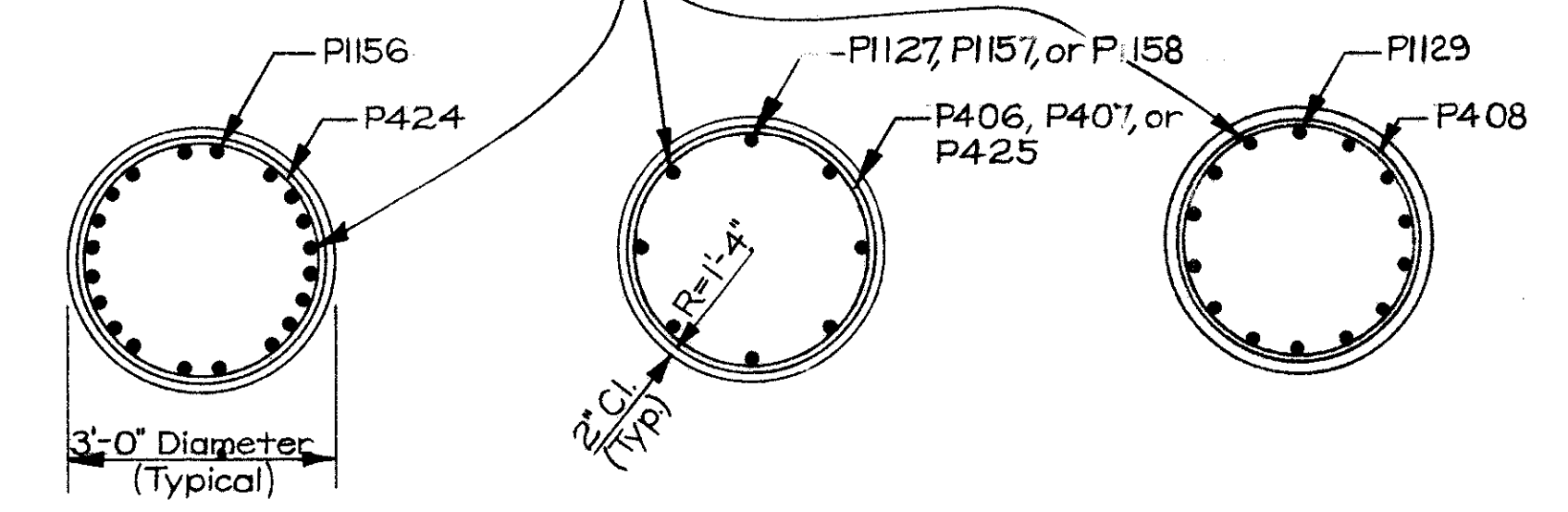
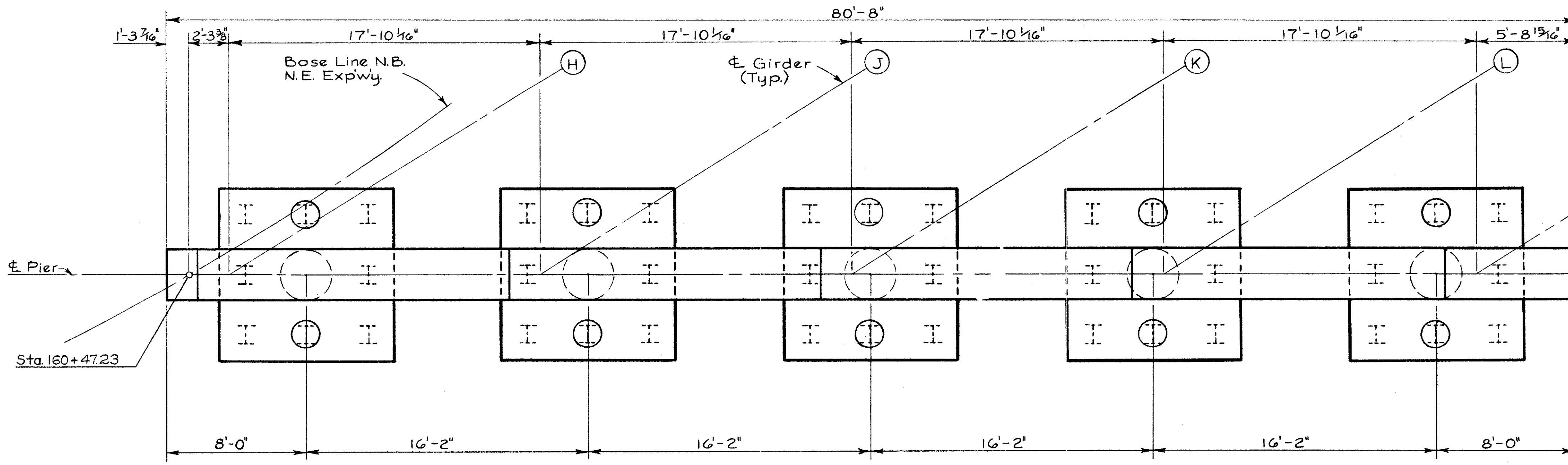
For footing details and reinforcing steel see Footing I-A, Sheet No. 352

MICROFILMED
FEB 20 1985

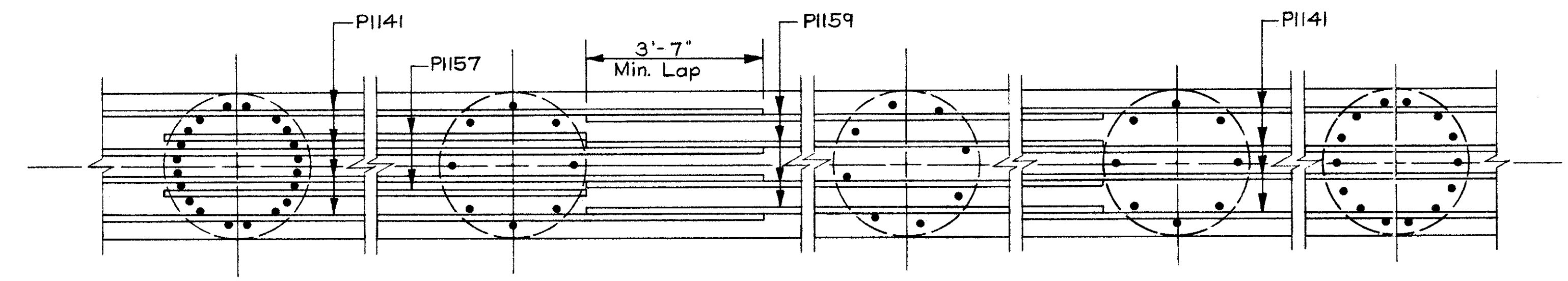
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
PIER NO. 1				
BRIDGE No HAM-71-0173				
H&E BRIDGE No. 8				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
JDC.	JDC.	JDC.	JDC.	H.A.B. 5-20-65
				REVISION

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

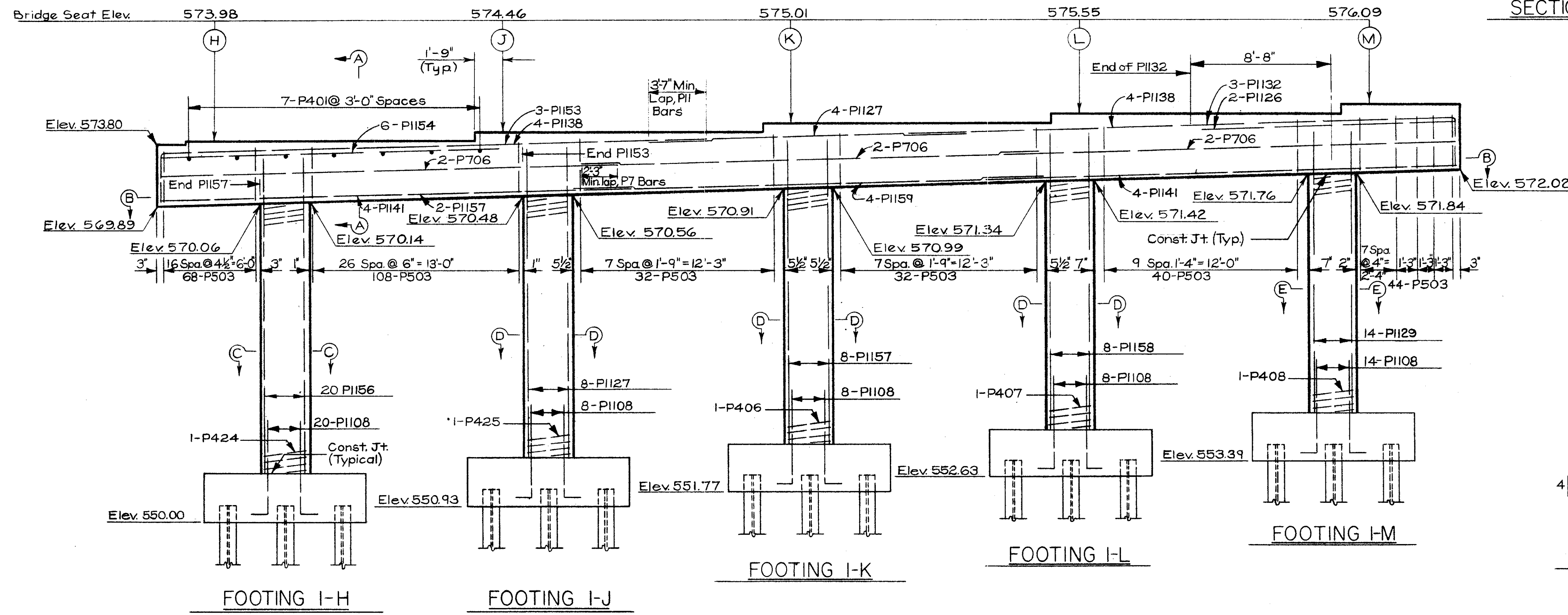
Note: Bars are to be spaced to match bottom cap reinforcing as dimensioned in Section A-A.



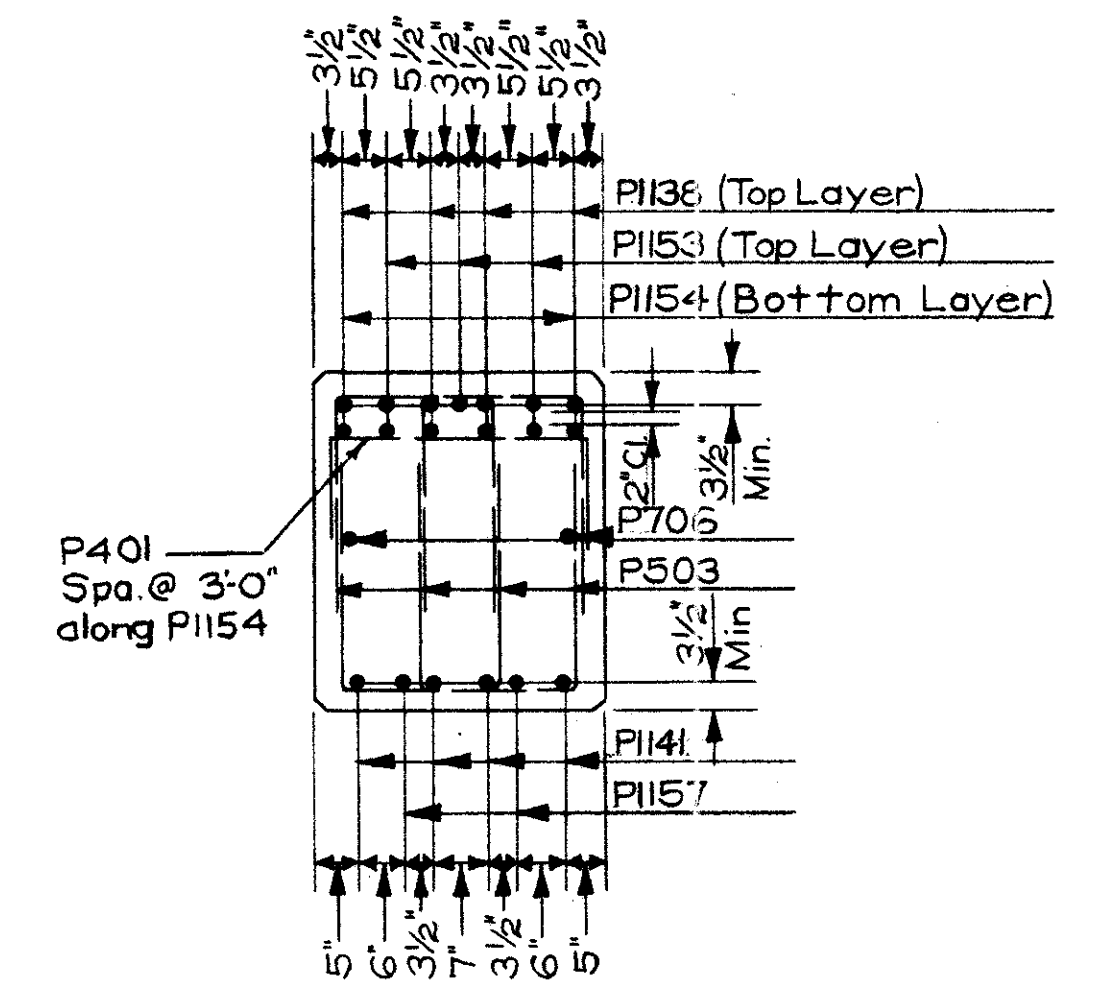
PLAN



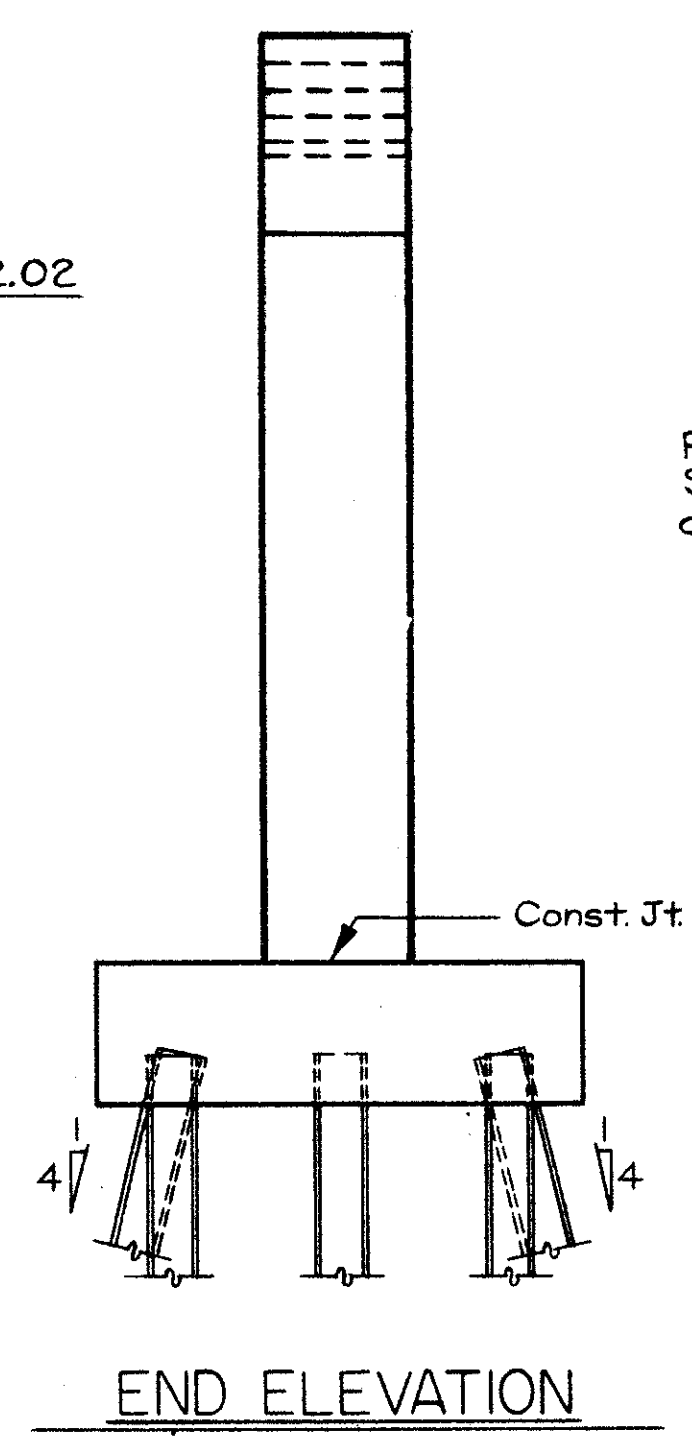
SECTION B-B



ELEVATION



SECTION A-A



END ELEVATION

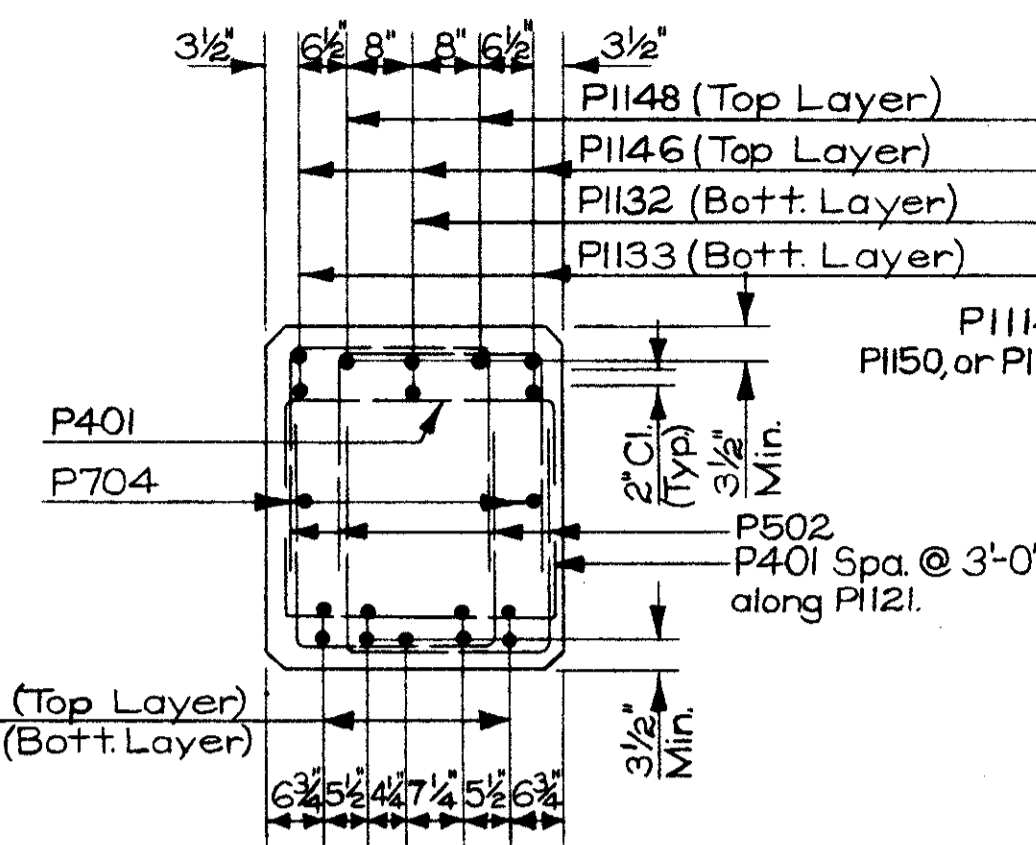
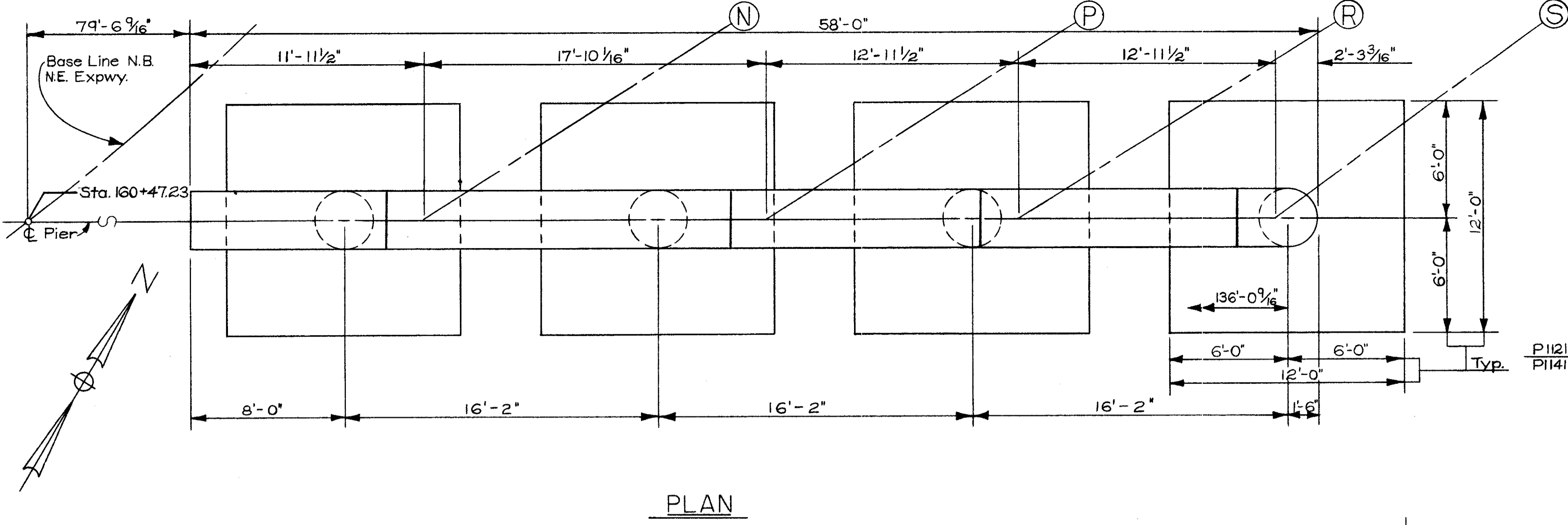
For footing details and reinforcing steel see Footing I-A, Sh.No. 352

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
PIER NO. 1					
BRIDGE No. HAM-71-0173					
H&E BRIDGE No. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	J.D.C.		J.L.S.	N.A.Z. 5-20-65	

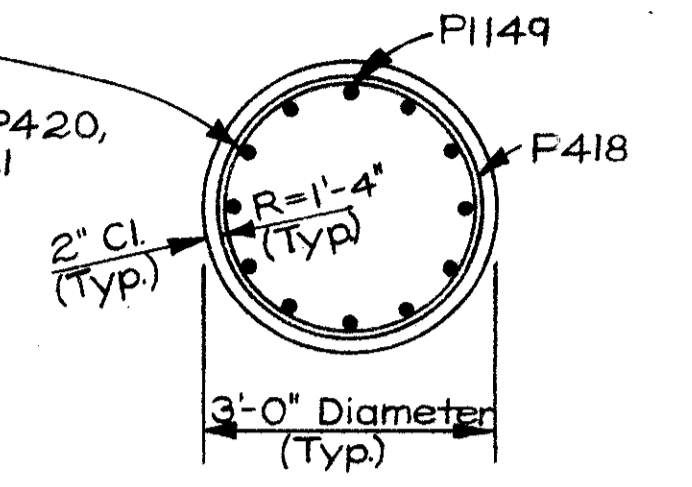
MICROFILMED
FEB. 20 1985

HAMILTON COUNTY
HAM-71-(1.5G) (2.5I)

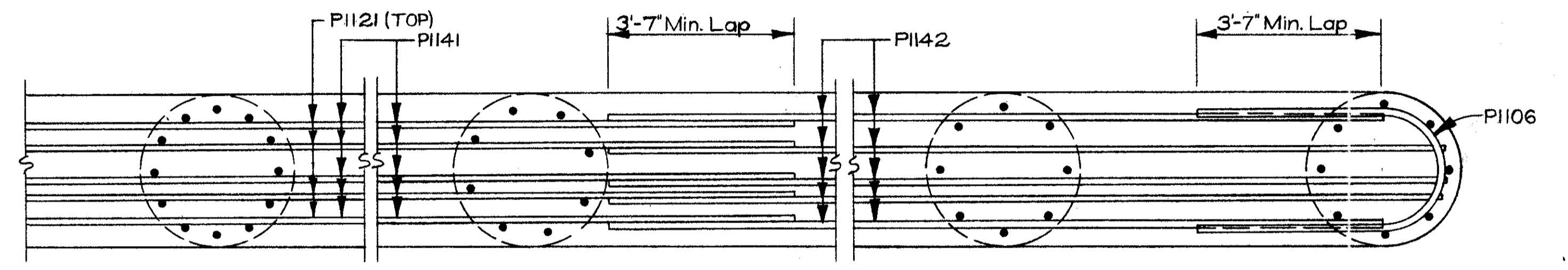
Note: Bars are to be spaced to match bottom cap reinforcing as dimensioned in Section A-A.



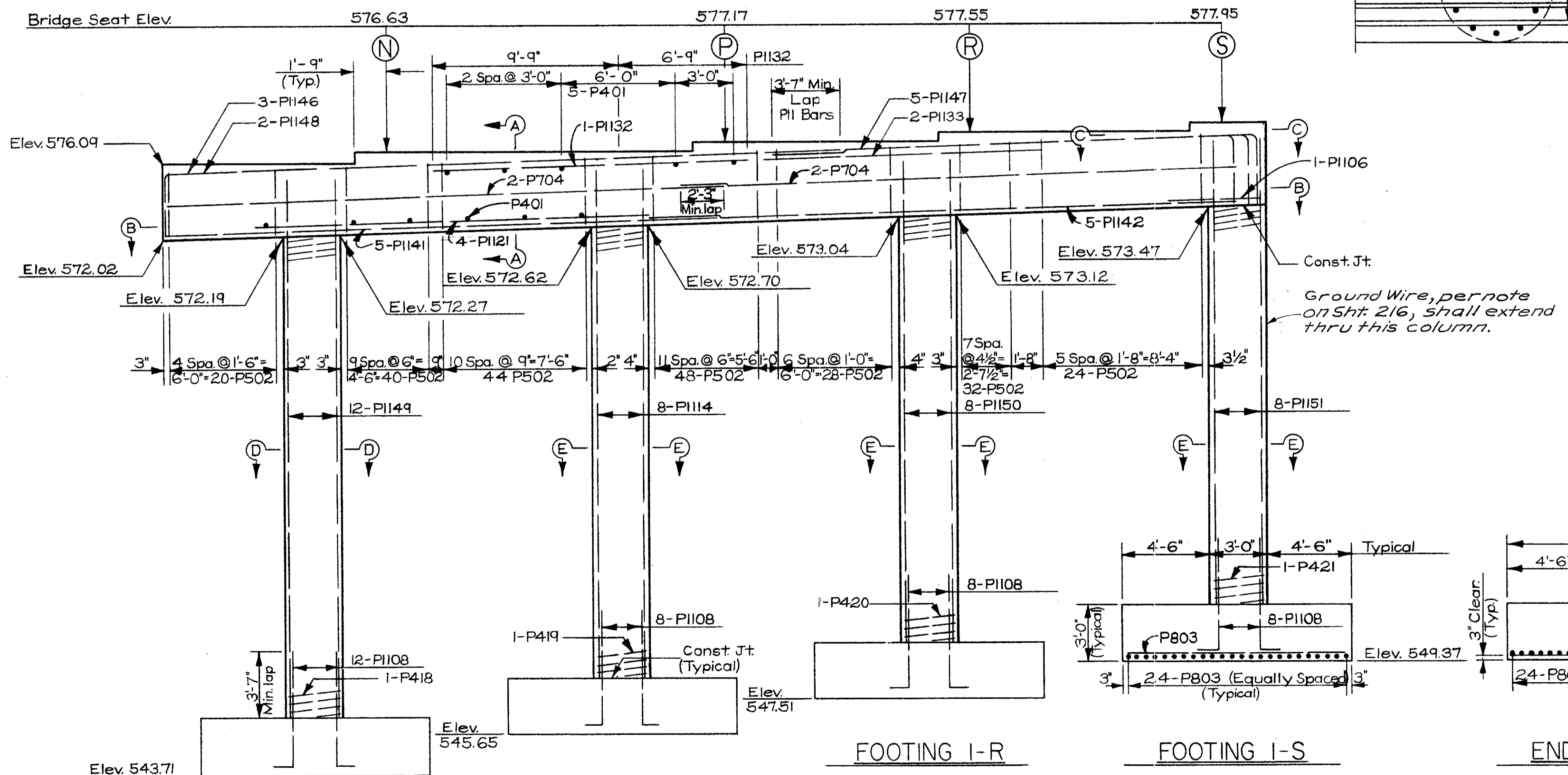
SECTION E E



SECTION D D



SECTION B B

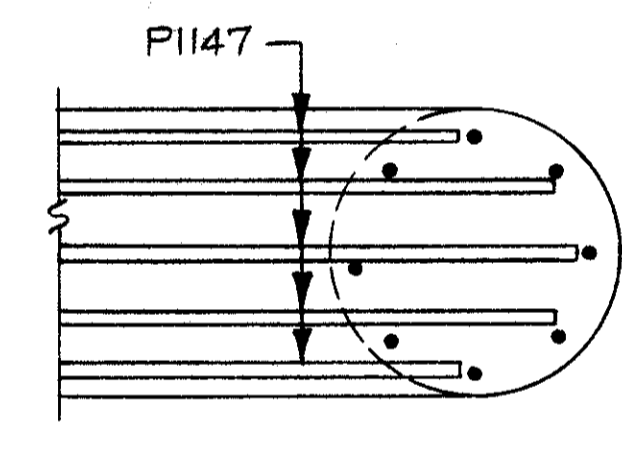


FOOTING I-P
ELEVATION

FOOTING I-R

FOOTING I-S

END ELEVATION



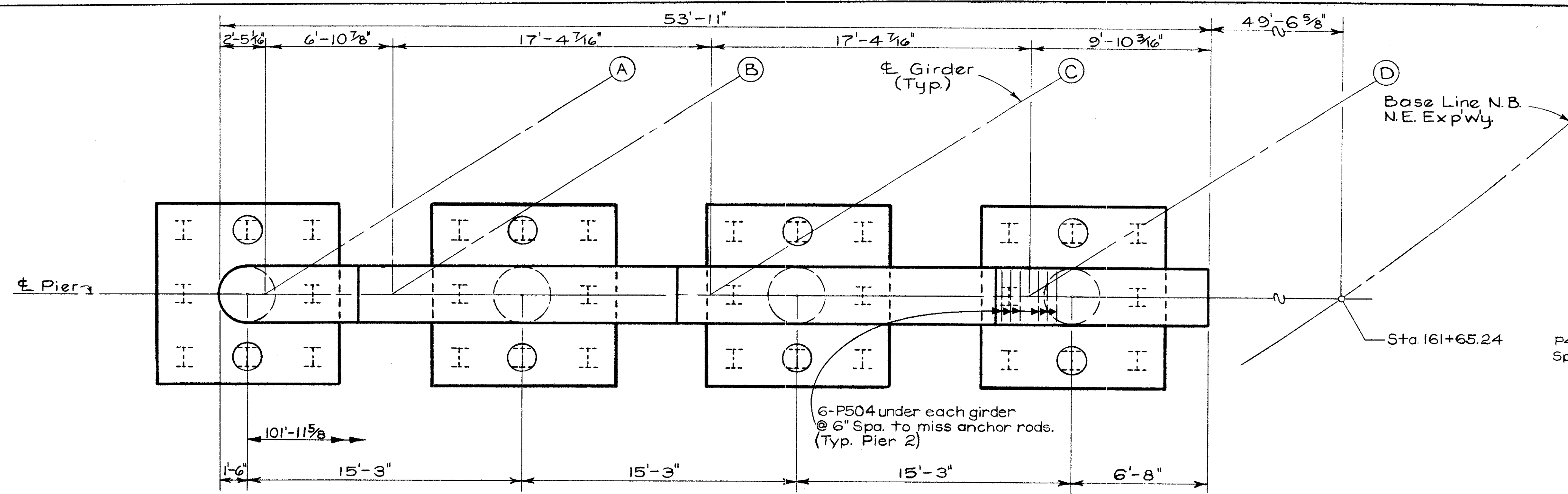
SECTION C C

FOOTINGS shall extend a minimum of 3' into undisturbed rock or to the elevation shown, whichever is lower.

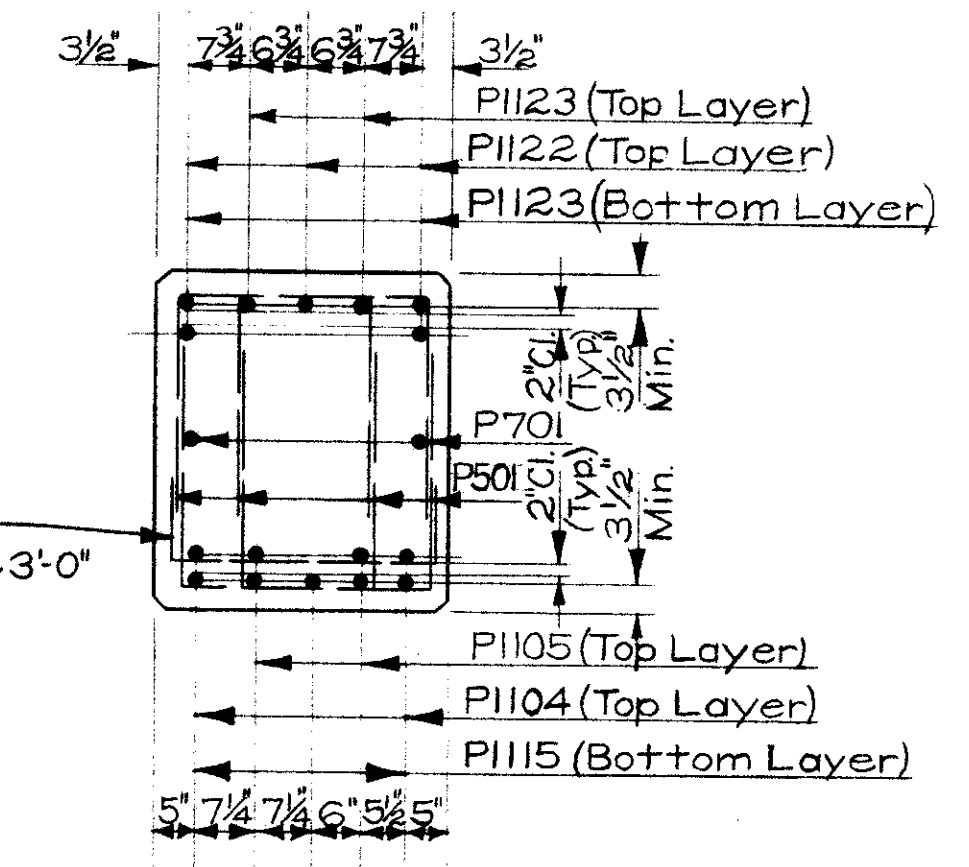
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
PIER NO. 1					
BRIDGE No. HAM-71-0173					
H&E BRIDGE No. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	CIB		Jtk	N.A.B. 5-20-65	

MICROFILMED
FEB 20 1985

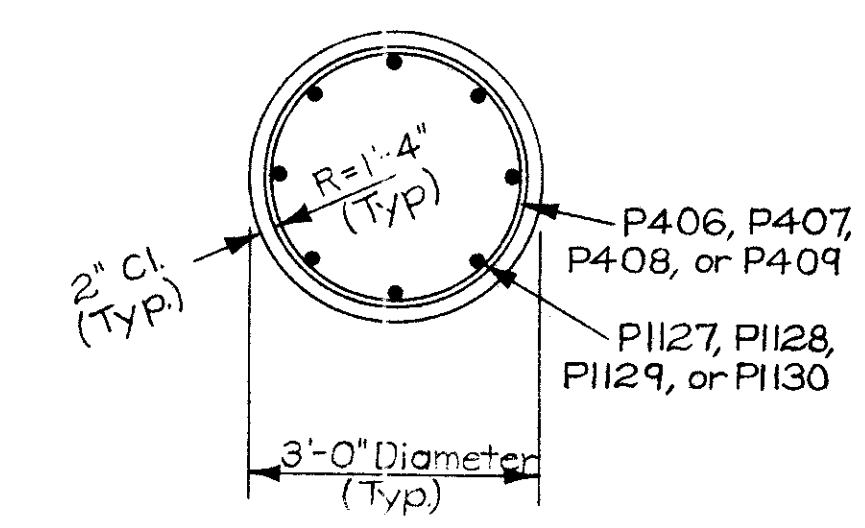
HAMILTON COUNTY
HAM-71-1156 (2.51)



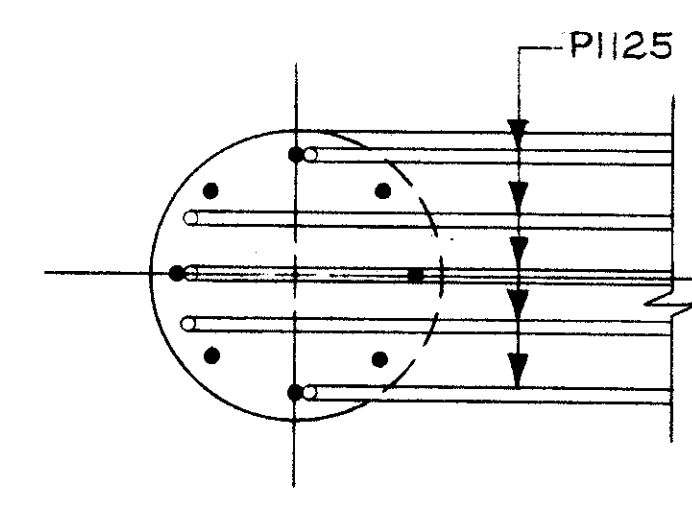
PLAN



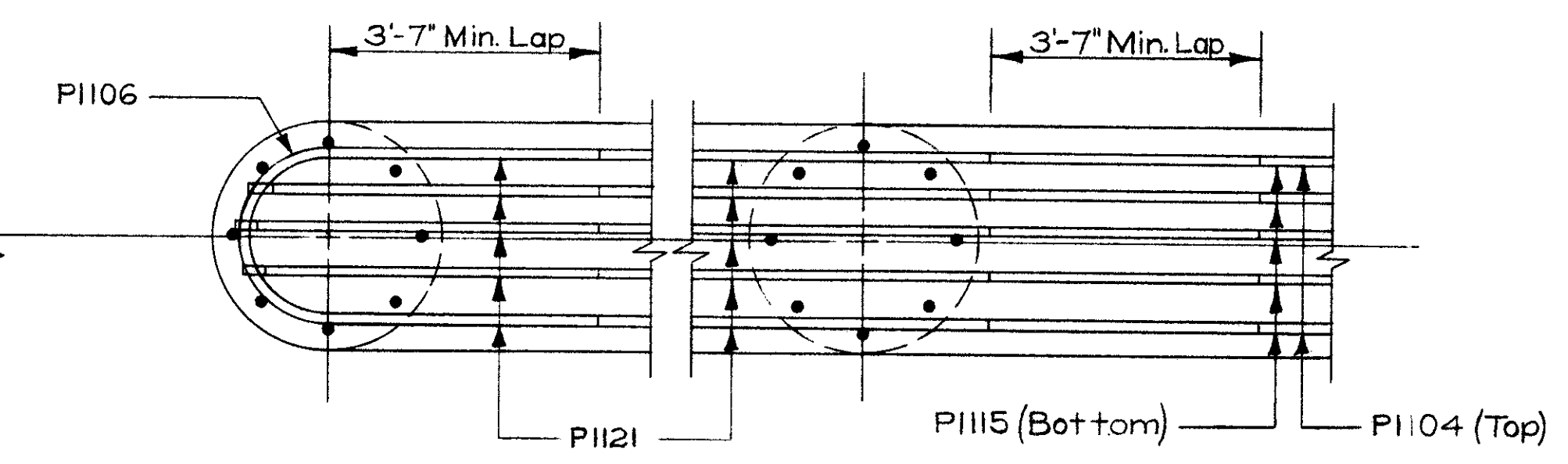
SECTION A-A



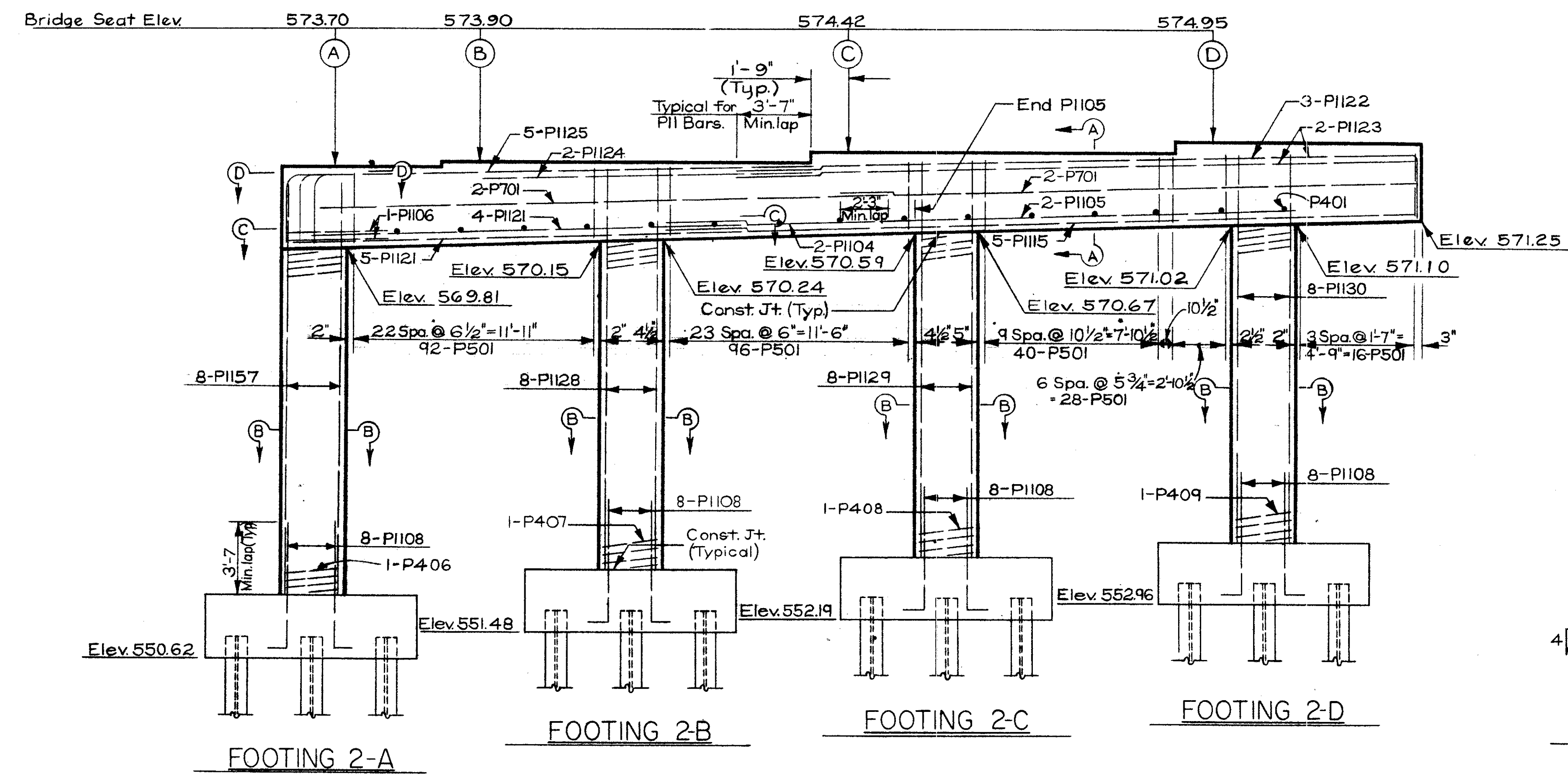
SECTION B-B



SECTION D-D

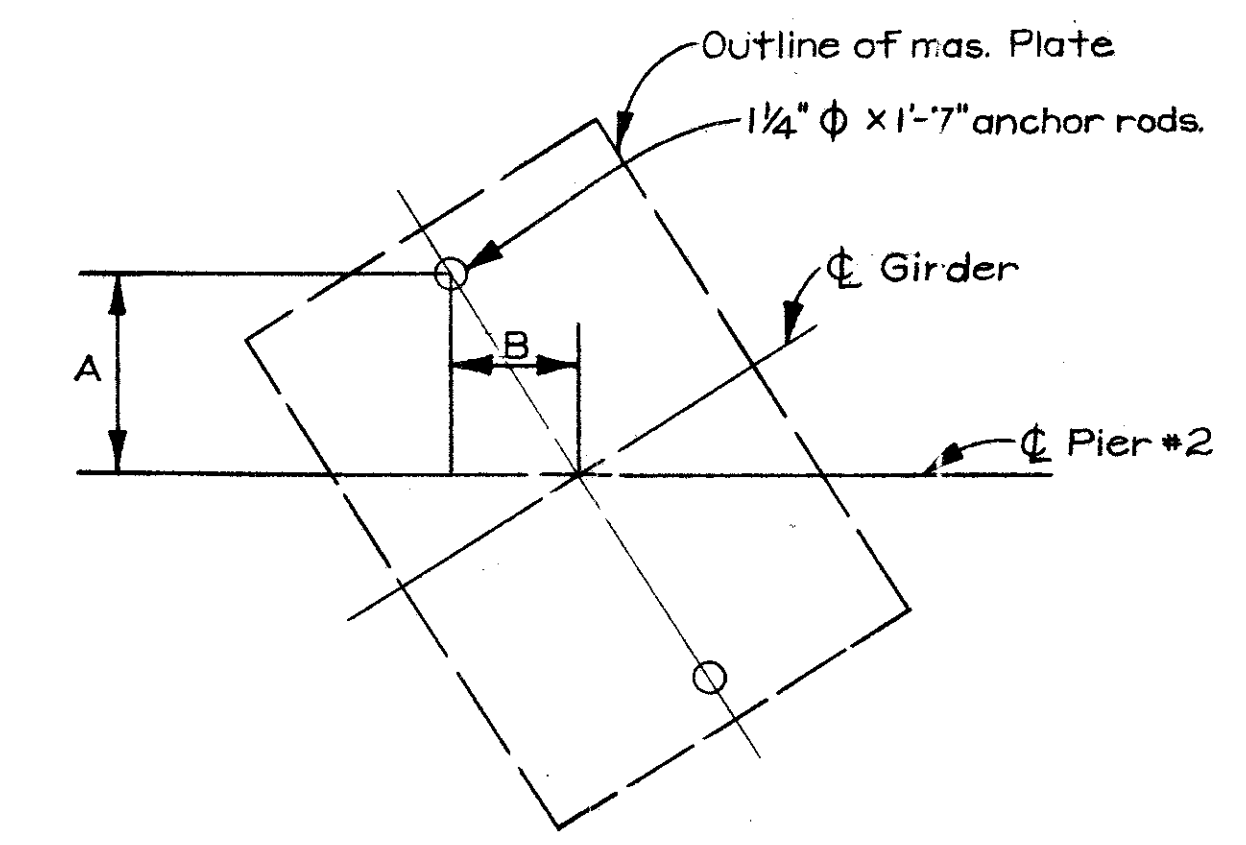


SECTION C-C

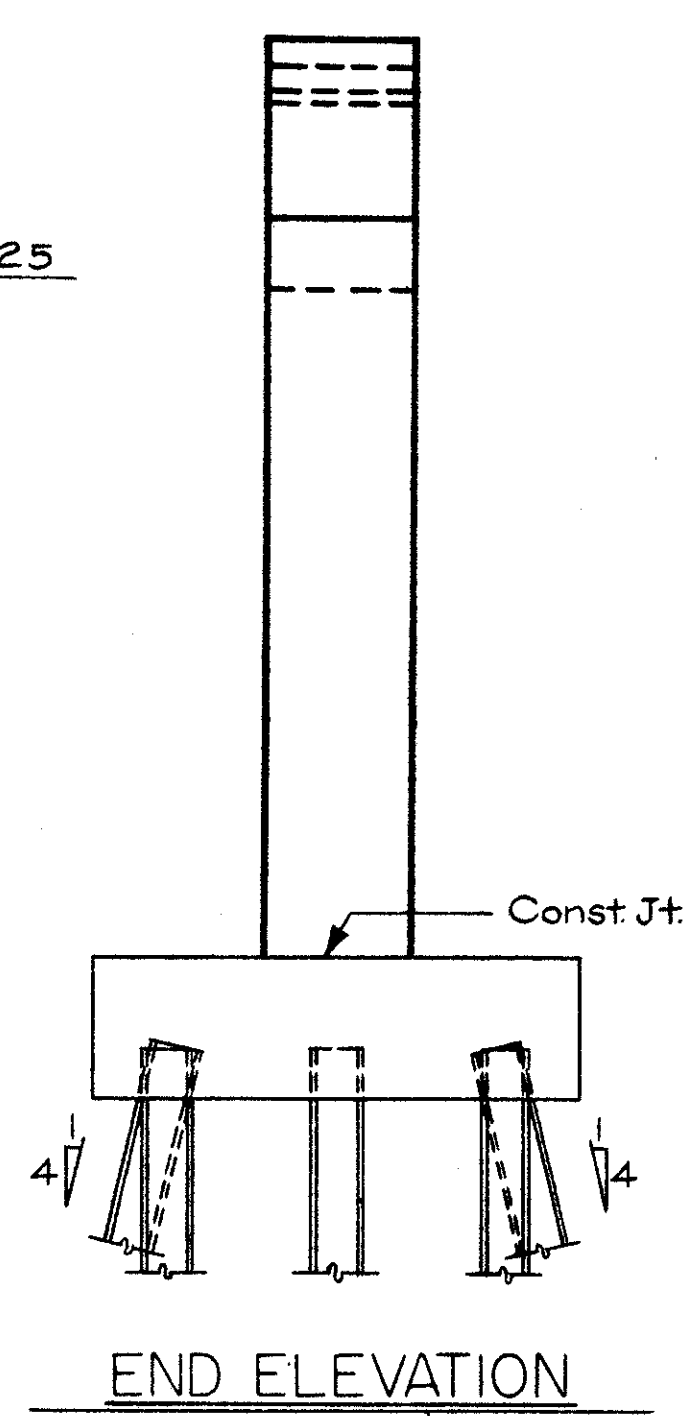


ELEVATION

Girder	A	B
A	8 7/16"	5 9/8"
B thru P	8 7/16"	5 5/16"
R	8 5/16"	5 9/16"
S	8 1/8"	5 7/8"



ANCHOR ROD LAYOUT



END ELEVATION

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FEB 20 1965

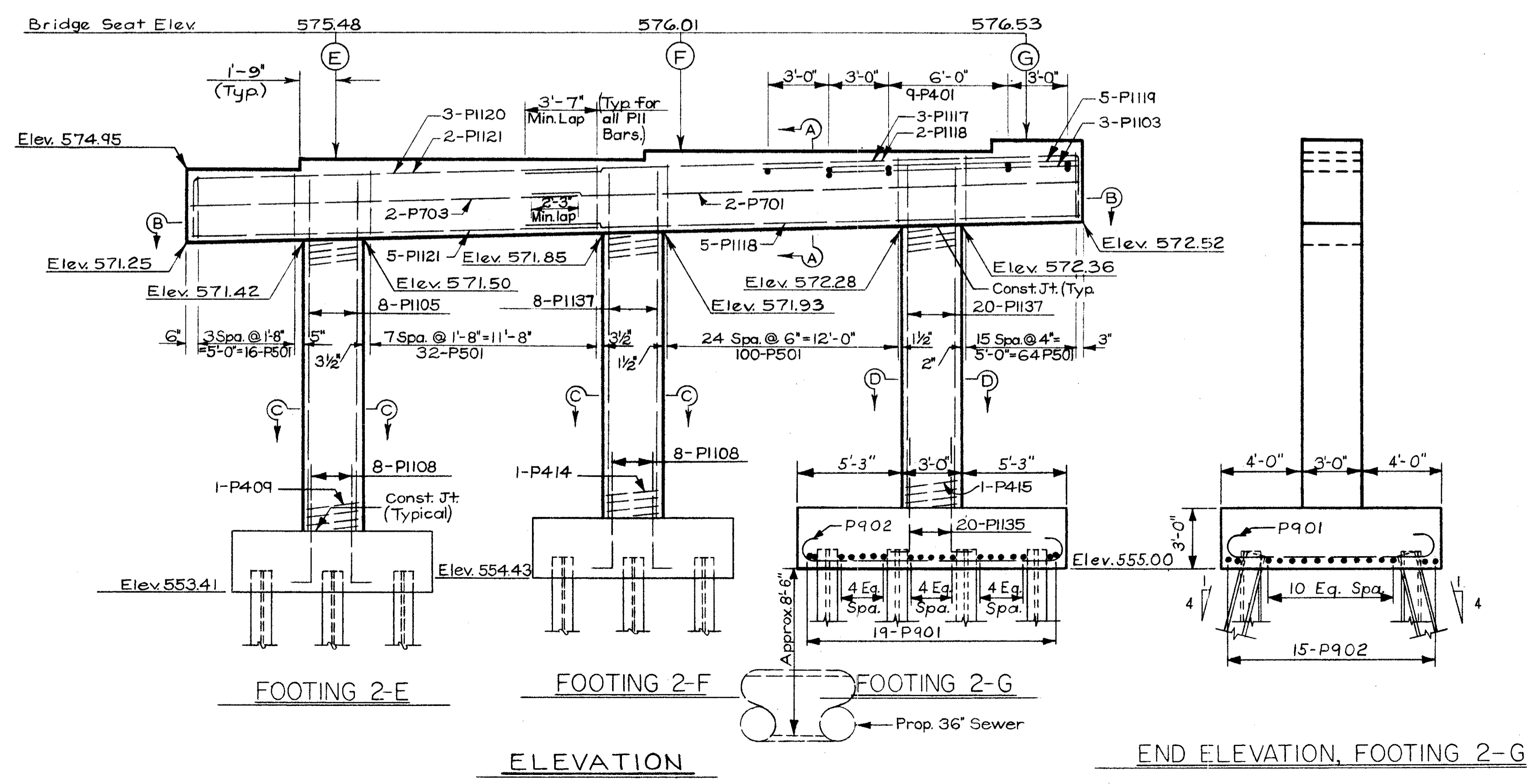
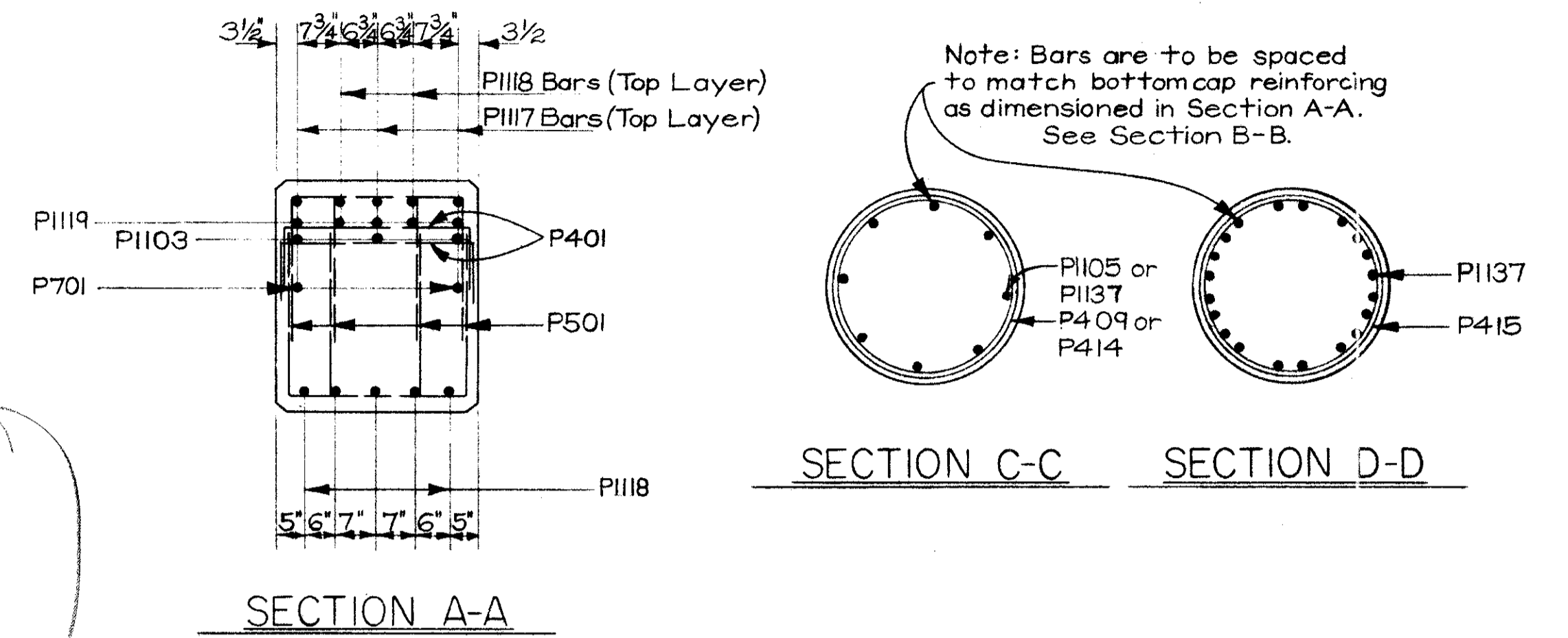
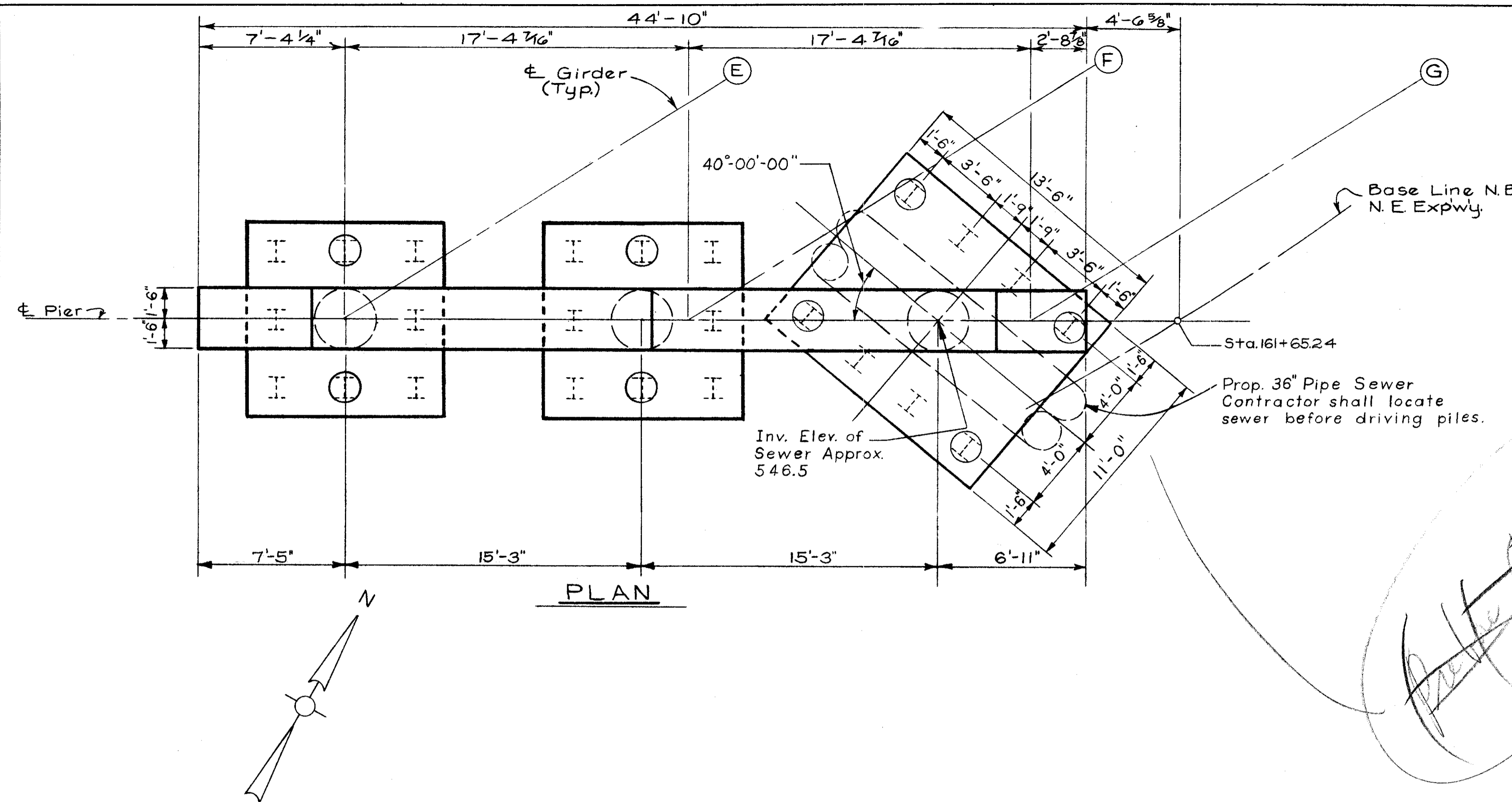
HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

PIER NO. 2
BRIDGE No. HAM-71-0173

H&E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	J.D.C.		J.H.O.	H.A.S. 5-20-65	

For Footing details and reinforcing steel see Footing I-A, Sheet 352



SECTION B-B

MICROFILMED
FEB 20 1965

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

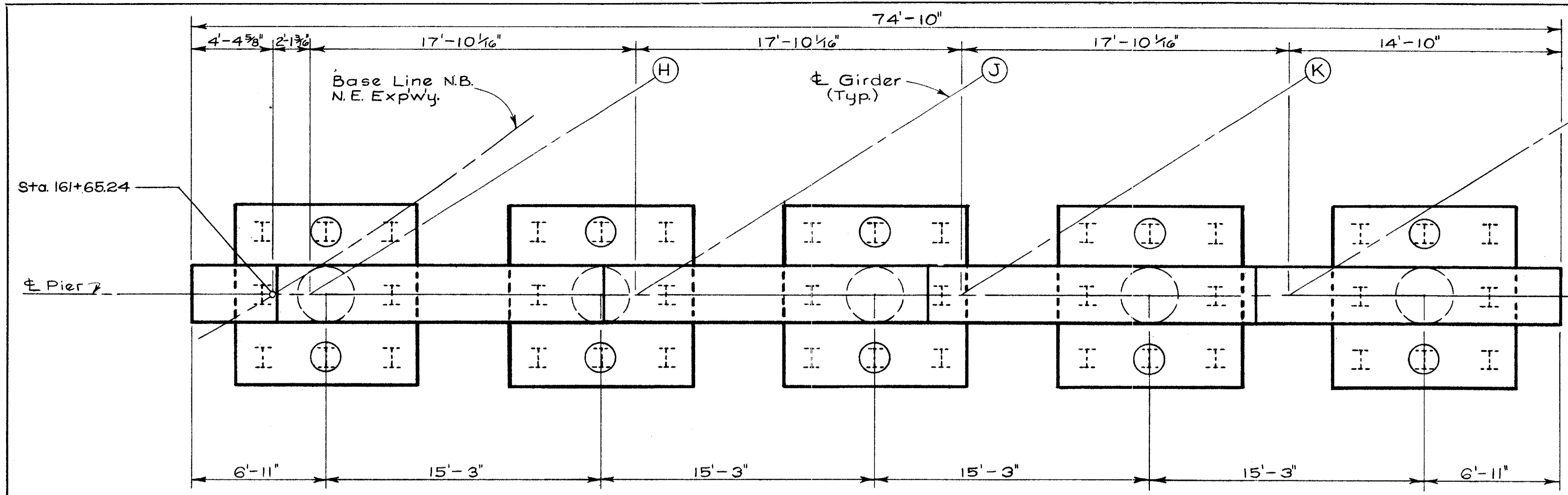
PIER NO. 2
BRIDGE No. HAM-71-0173

H&E BRIDGE No. 8

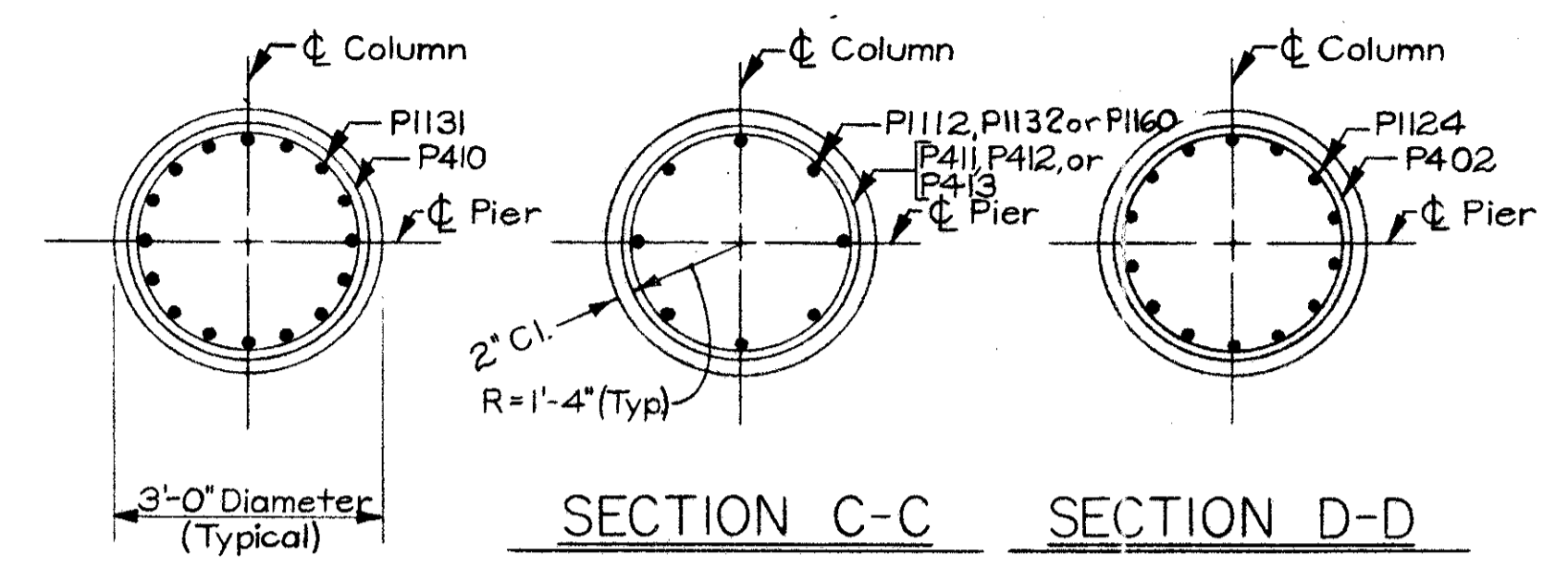
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
JDC.	J40		J40	11/18/65	
				5-20-65	

For footing details and reinforcing steel for Footings 2-E & 2-F see Footing 1-A, Sh.No. 352

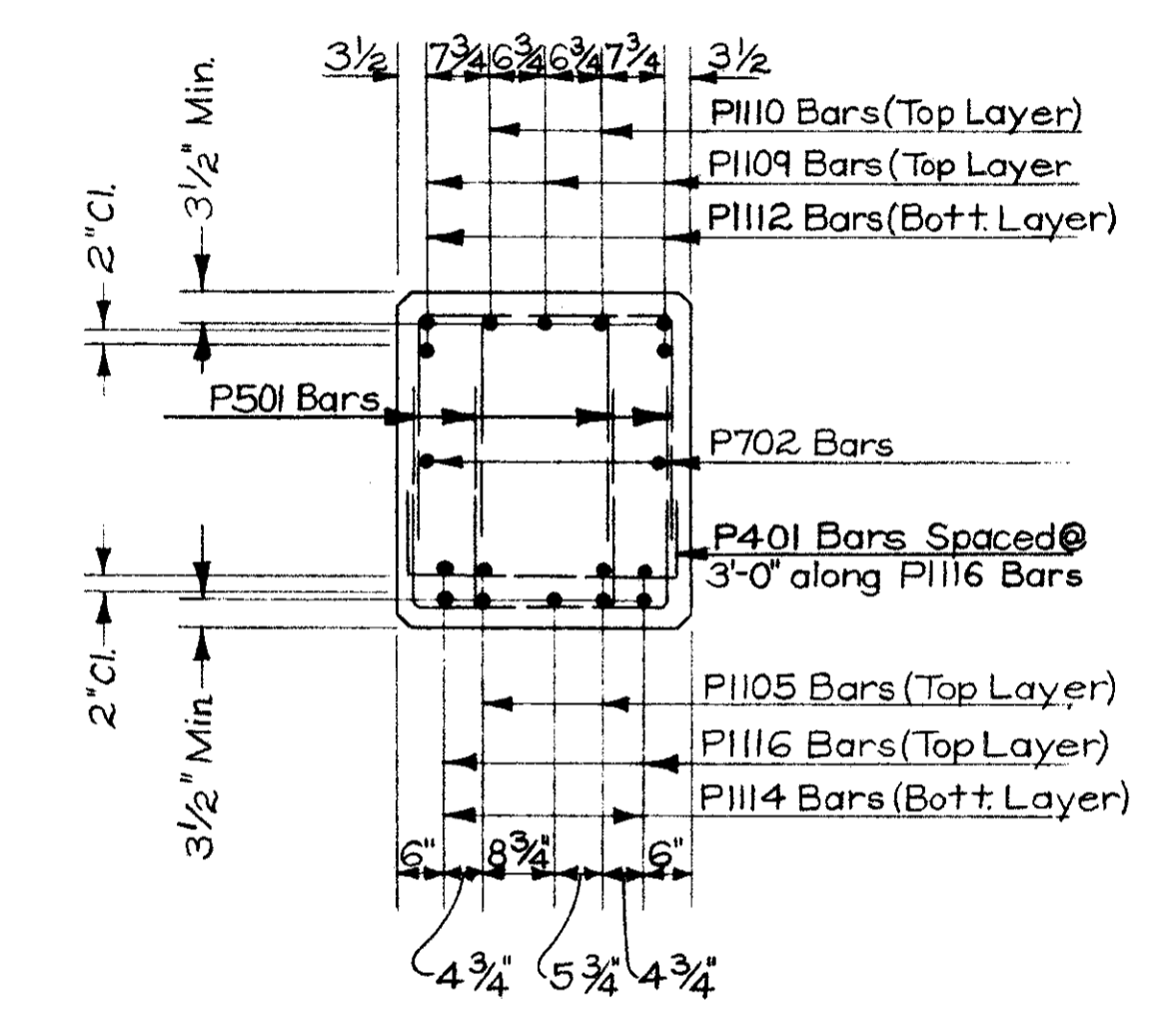
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



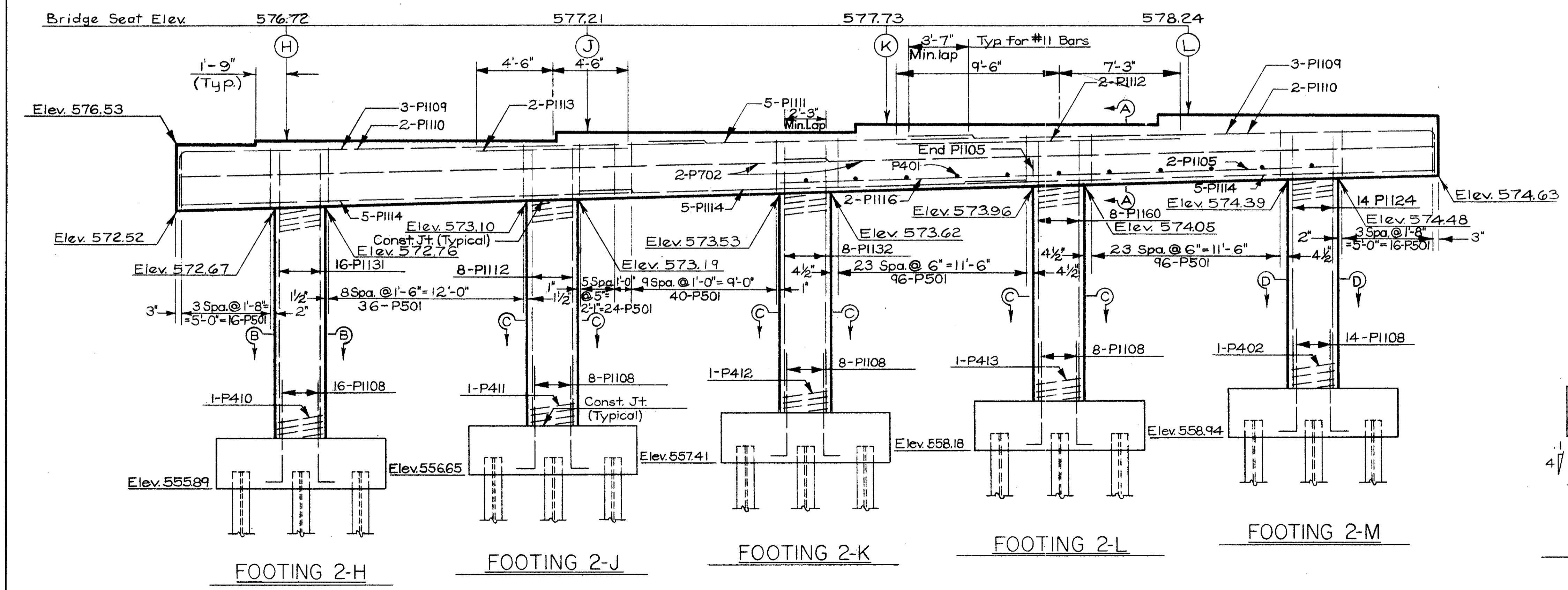
PLAN



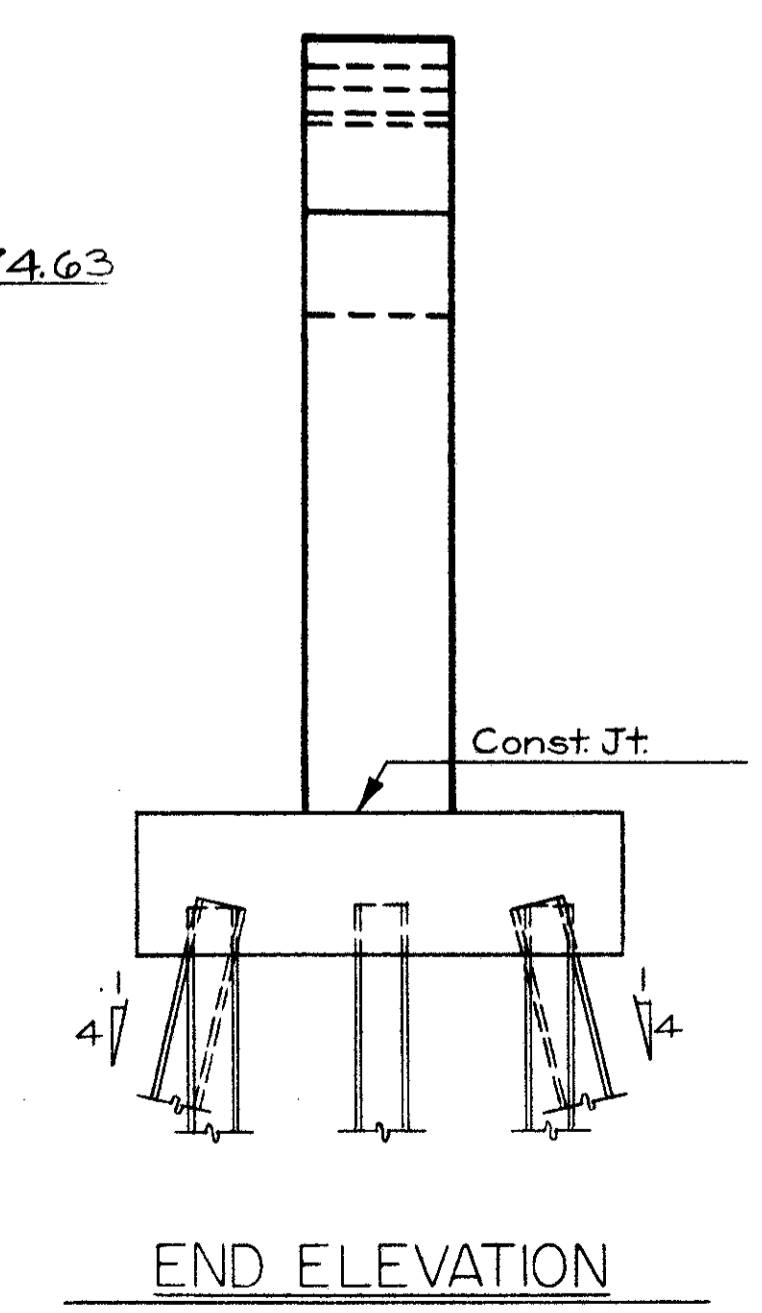
SECTION B-B



SECTION A-A



ELEVATION



END ELEVATION

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
PIER NO. 2				
BRIDGE No. HAM-71-0173				
H&E BRIDGE No. 8				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
J.D.C.	J.D.C.	J.D.C.	J.H.S.	5-20-65

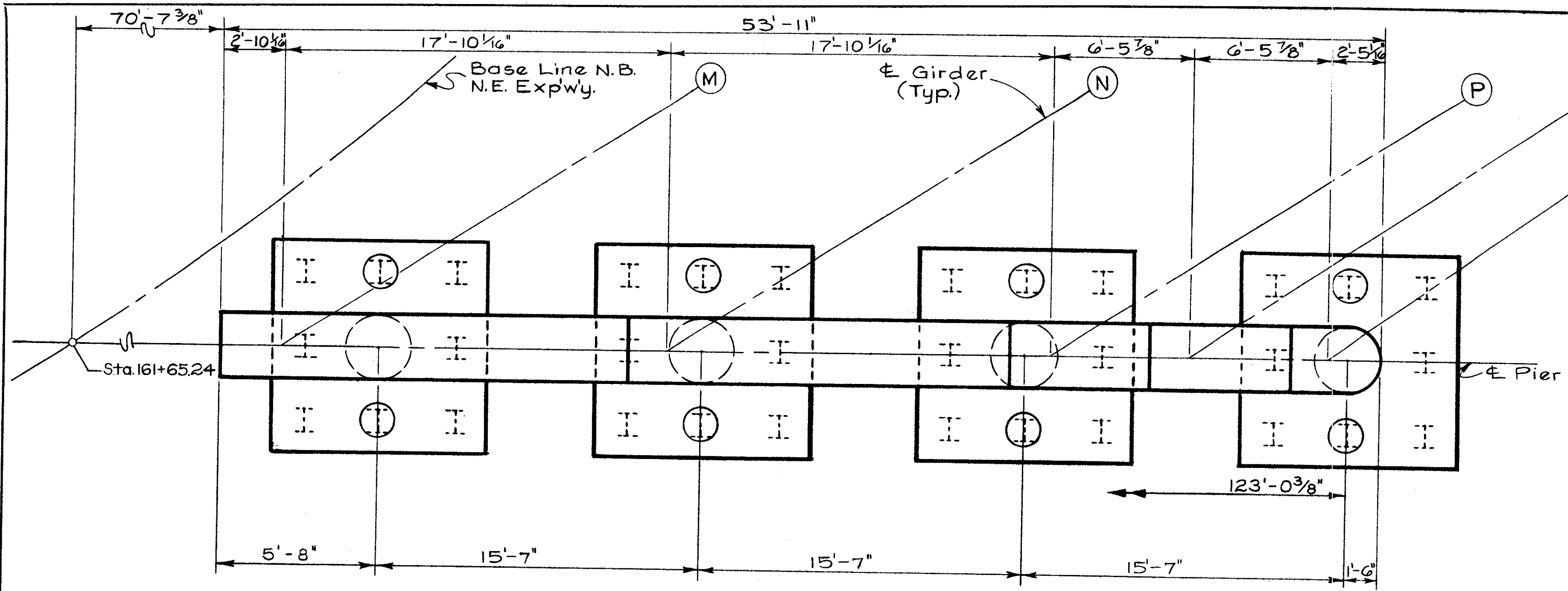
MICROFILMED
FEB 20 1985

For footing details and reinforcing steel see Footing 1-A, Sheet No. 352

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

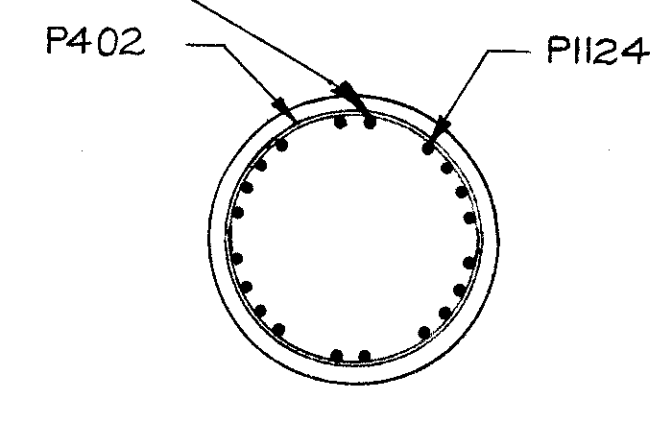
359
492

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

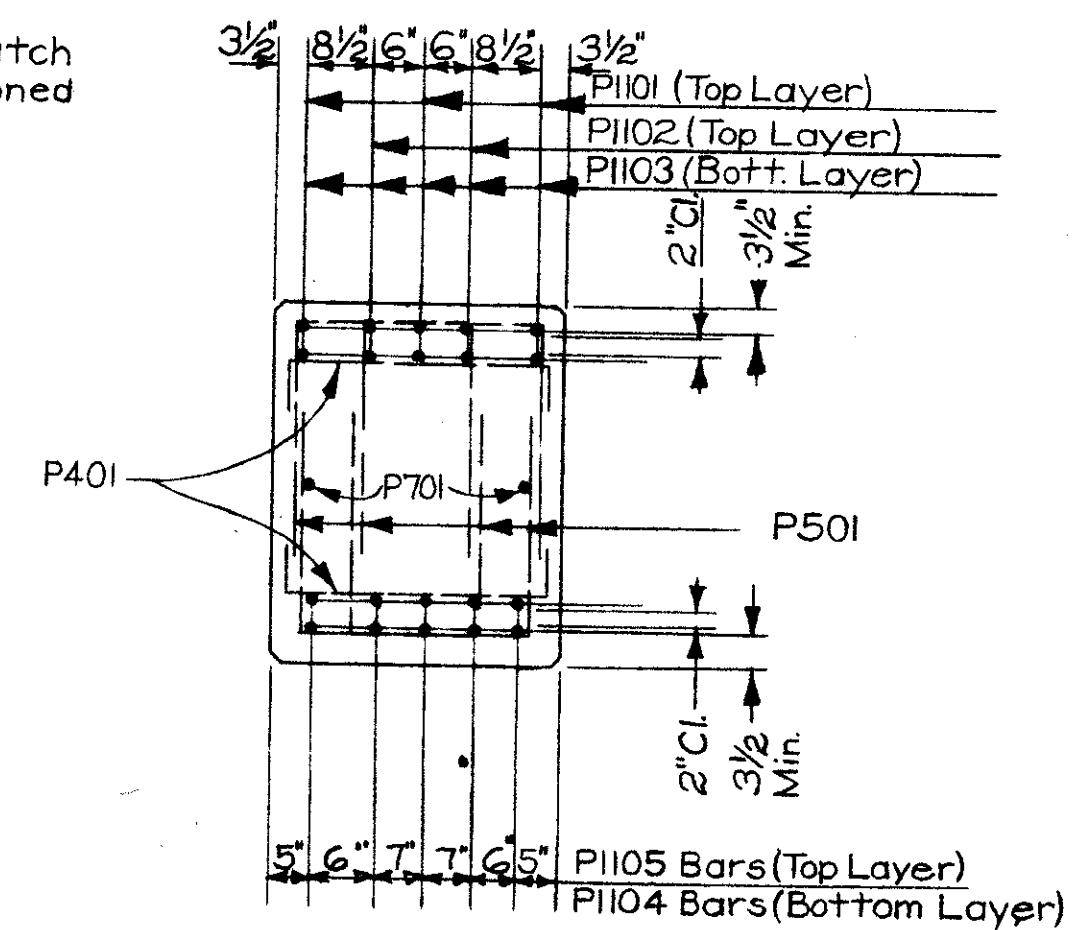


PLAN

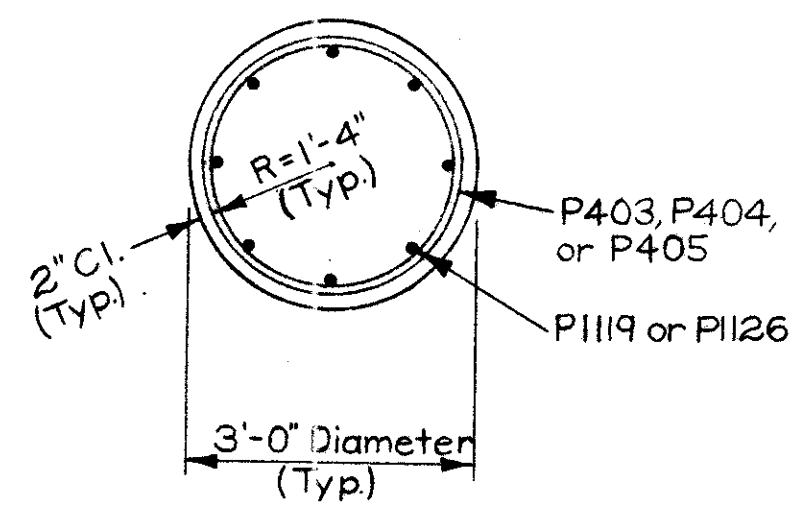
Note: Bars are to be spaced to match bottom cap reinforcing as dimensioned in Section A-A. See Section B-B.



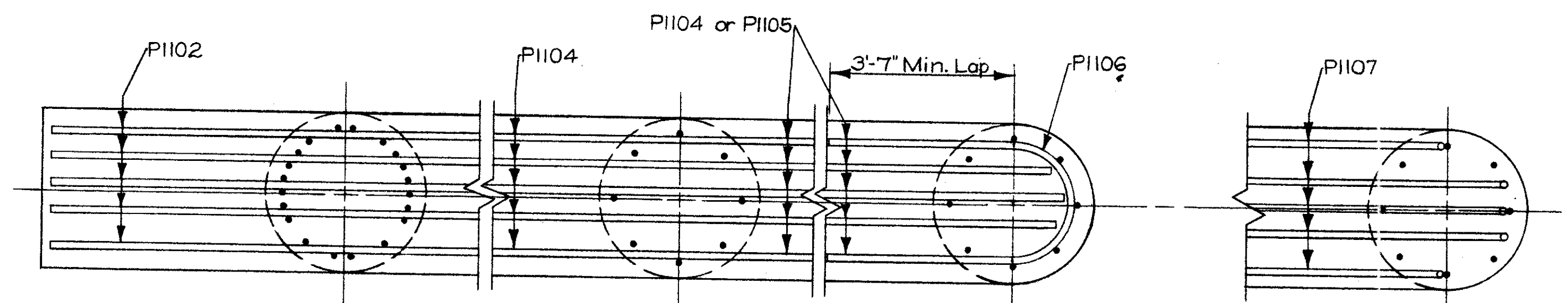
SECTION D-D



SECTION A-A

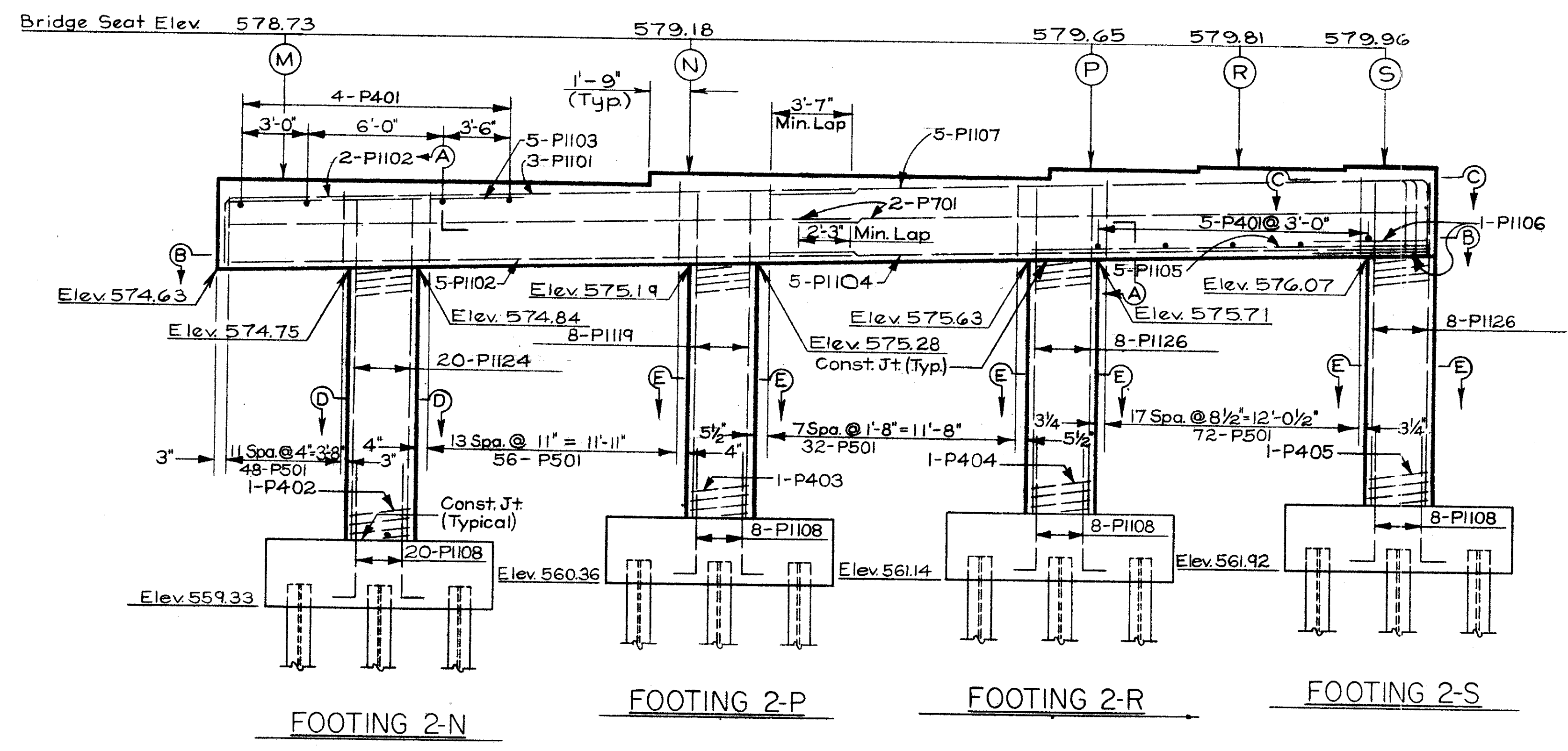


SECTION E-E

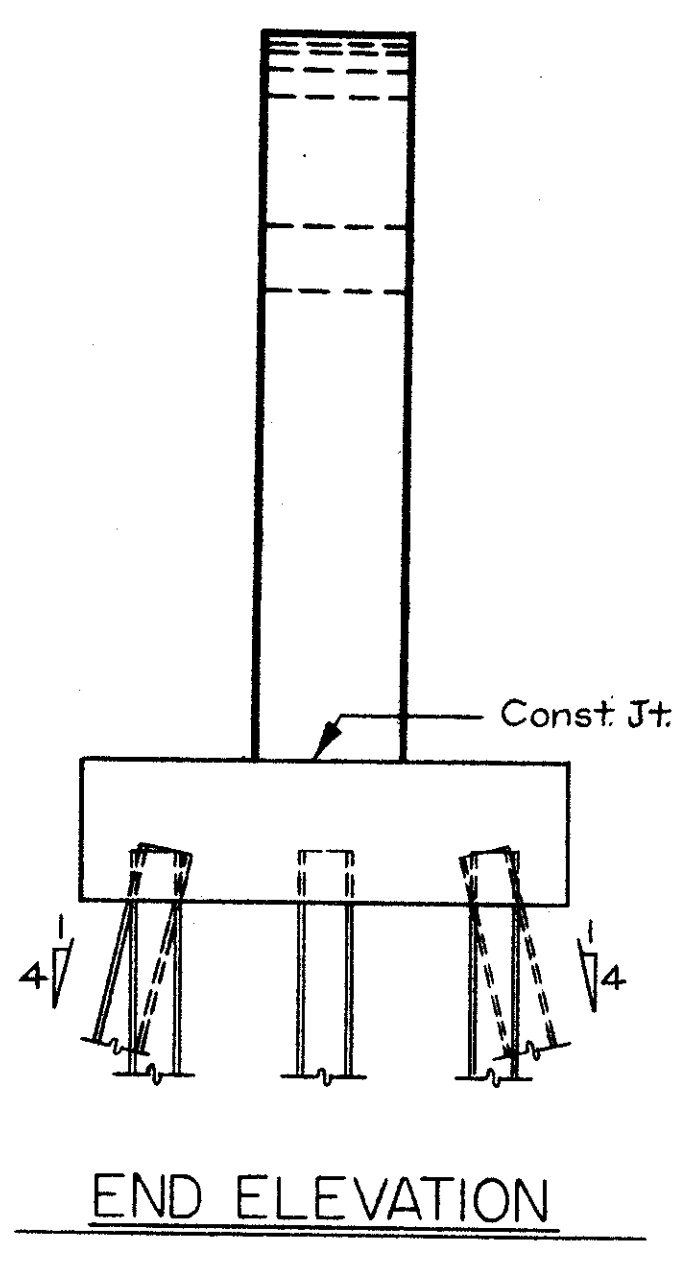


SECTION B-B

SECTION C-C



ELEVATION



END ELEVATION

MICROFILMED
FEB 20 1985

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

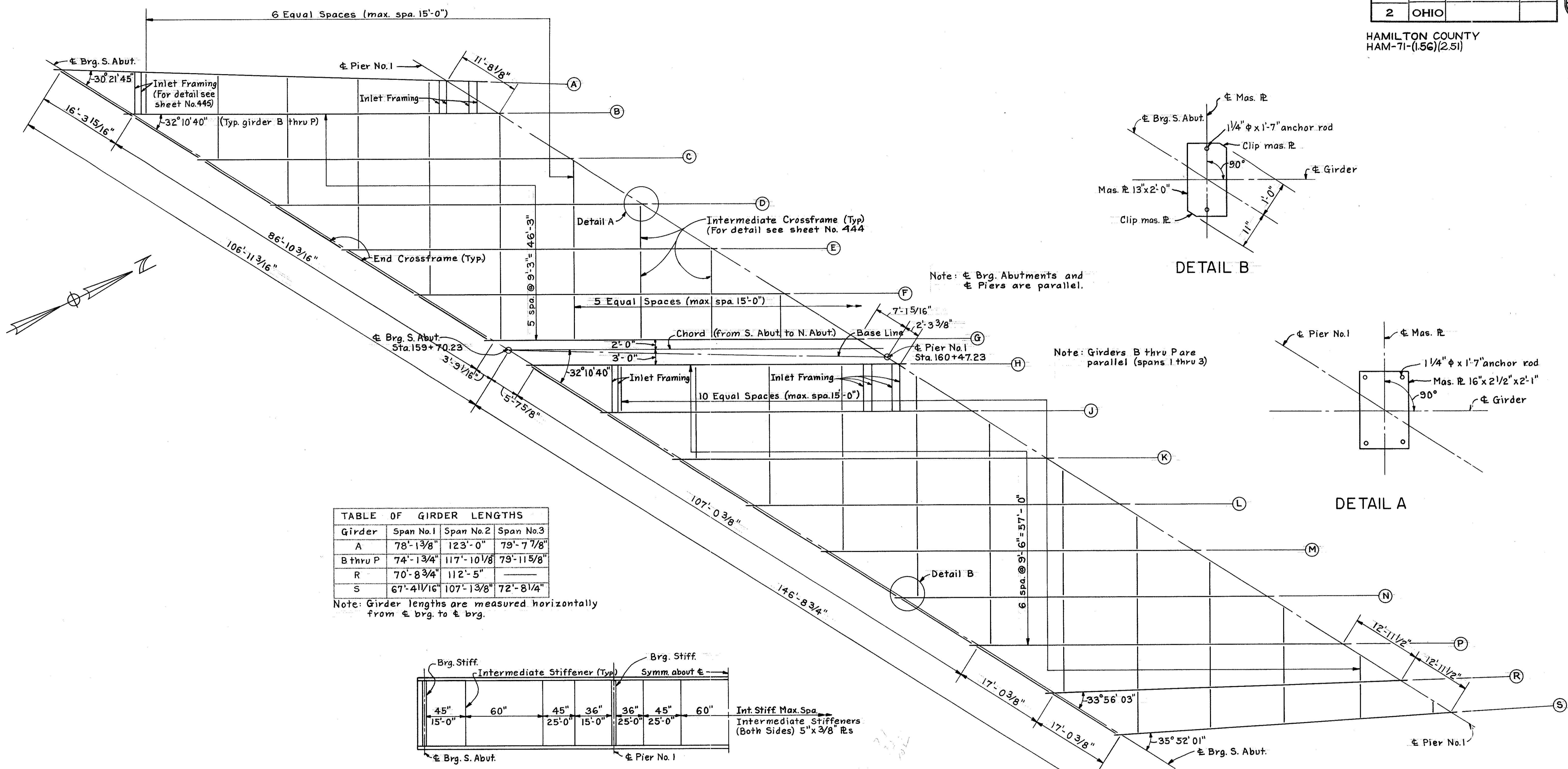
PIER NO. 2
BRIDGE No. HAM-71-0173

H&E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
J.D.C.	J.H.O.		J.H.O.	N.A.B. 5-20-65	

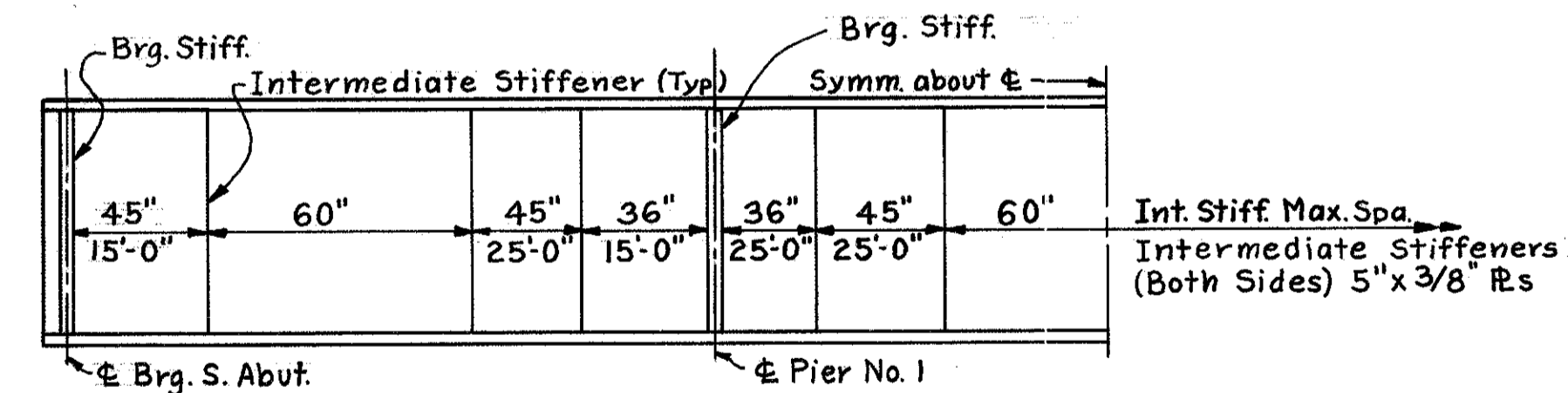
For footing details and reinforcing steel see Footing I-A Sheet 352

HAMILTON COUNTY
HAM-71-(156)(2.51)

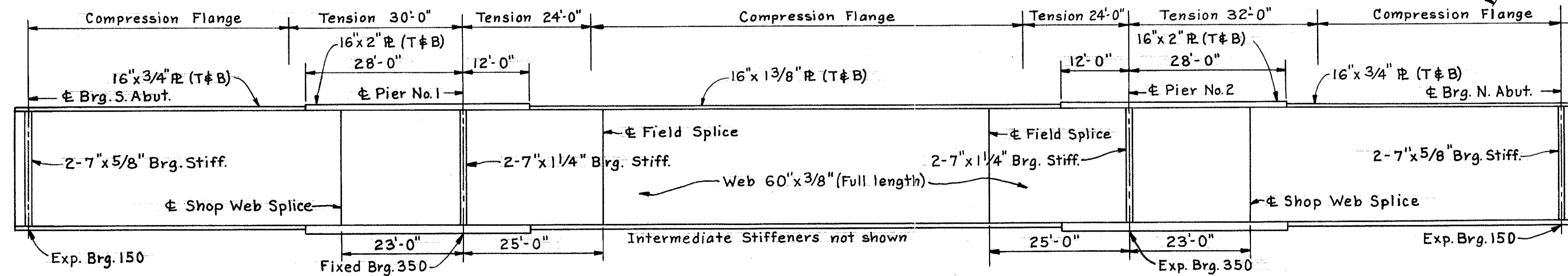


Girder	Span No.1	Span No.2	Span No.3
A	78'-1 3/8"	123'-0"	79'-7 7/8"
B thru P	74'-1 3/4"	117'-10 1/8"	79'-11 5/8"
R	70'-8 3/4"	112'-5"	
S	67'-4 1/16"	107'-1 3/8"	72'-8 1/4"

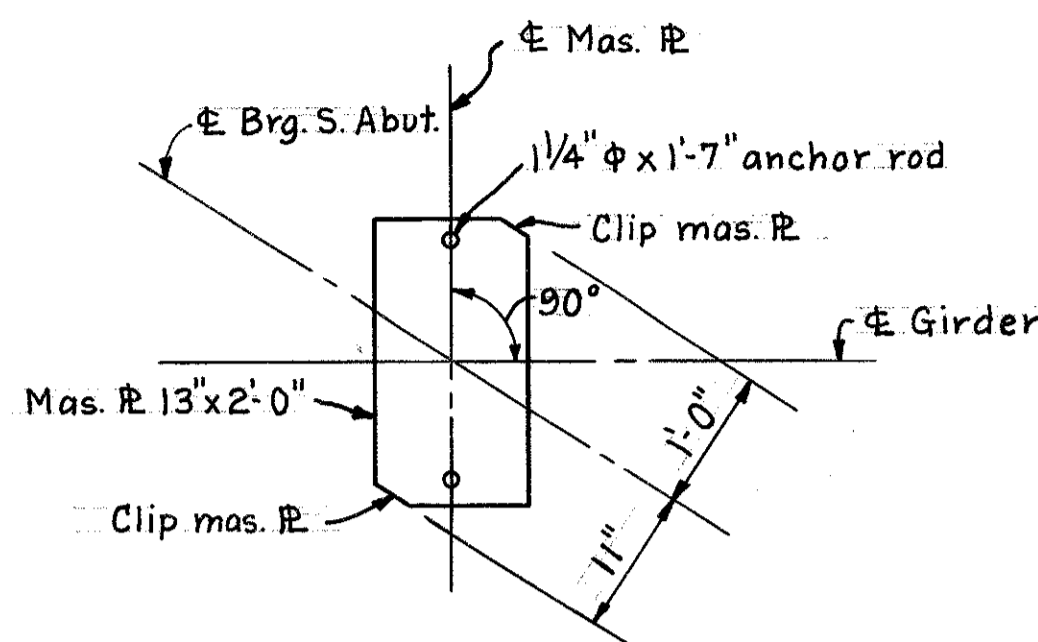
Note: Girder lengths are measured horizontally from ϕ brg. to ϕ brg.



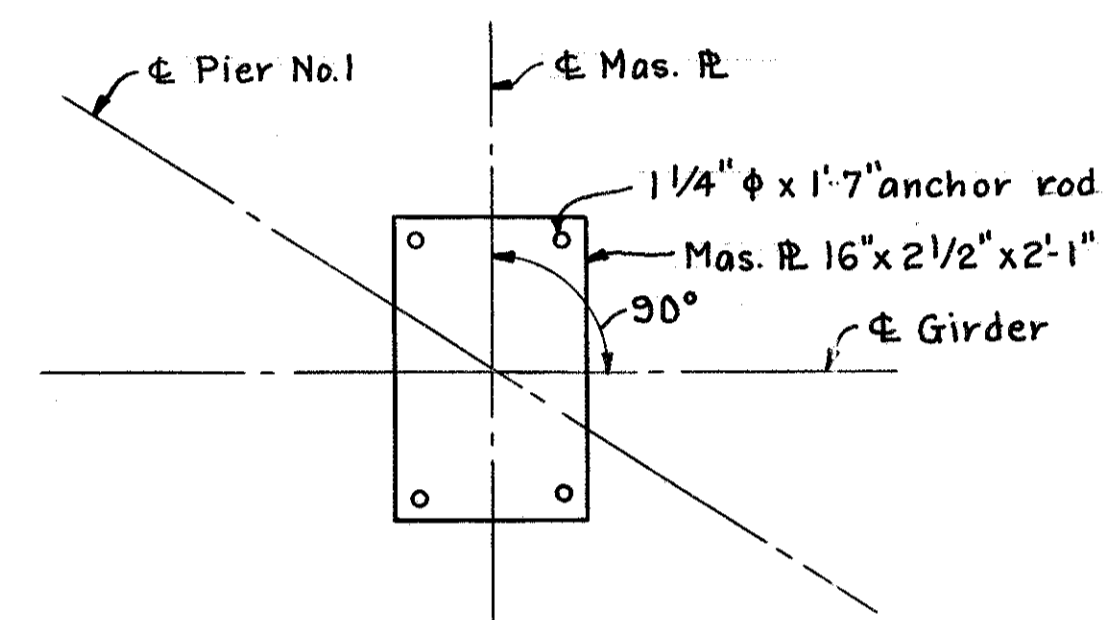
INTERMEDIATE STIFFENER SPACING



GIRDER ELEVATION



DETAIL B



DETAIL A

Notes:
For bearing details see drwg. No. FSB-1-62
For typical plate girder elevation see sheet No. 444
For detail of roadway and curb plate end dam details see Sh. 363
For treatment of ends of bridge see drwg. No. SD-1-63, sheet No. 2 "Longitudinal Section"

Work this sheet with sheet Nos. 361 & 362

HAZELT & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

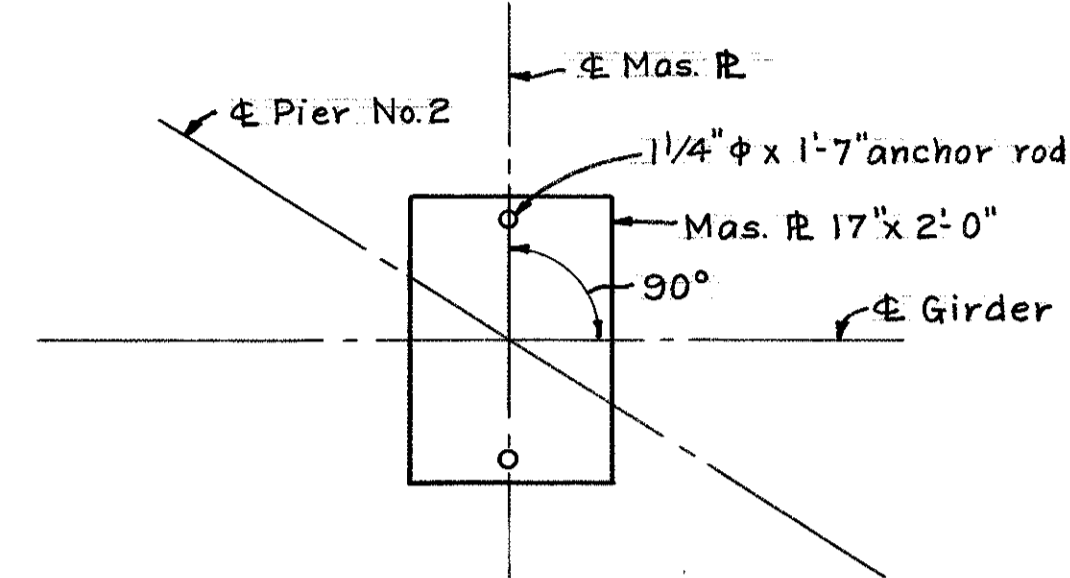
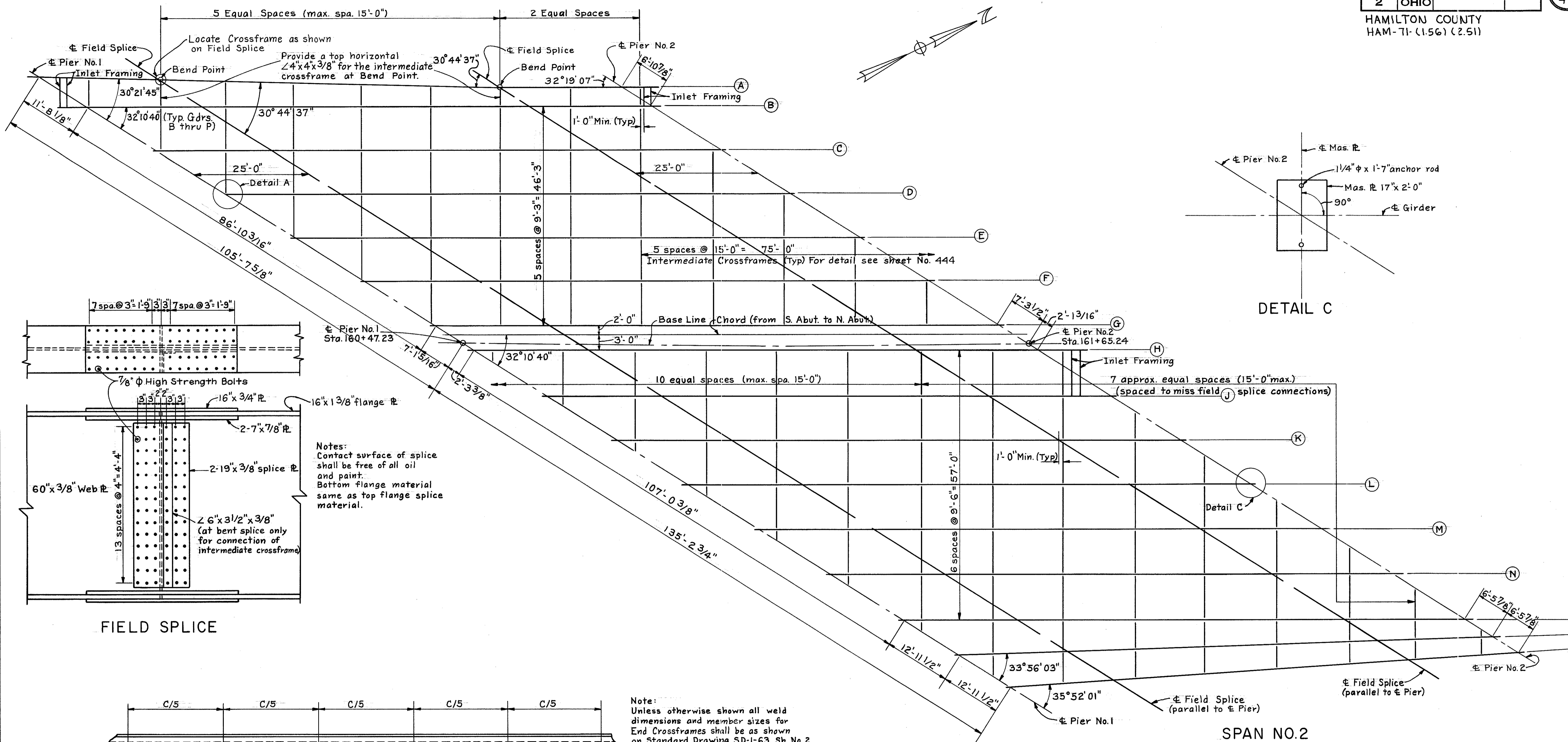
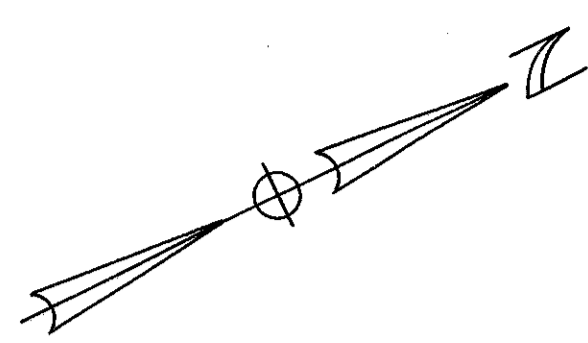
STRUCTURAL STEEL DETAILS

BRIDGE No. HAM-71-0173

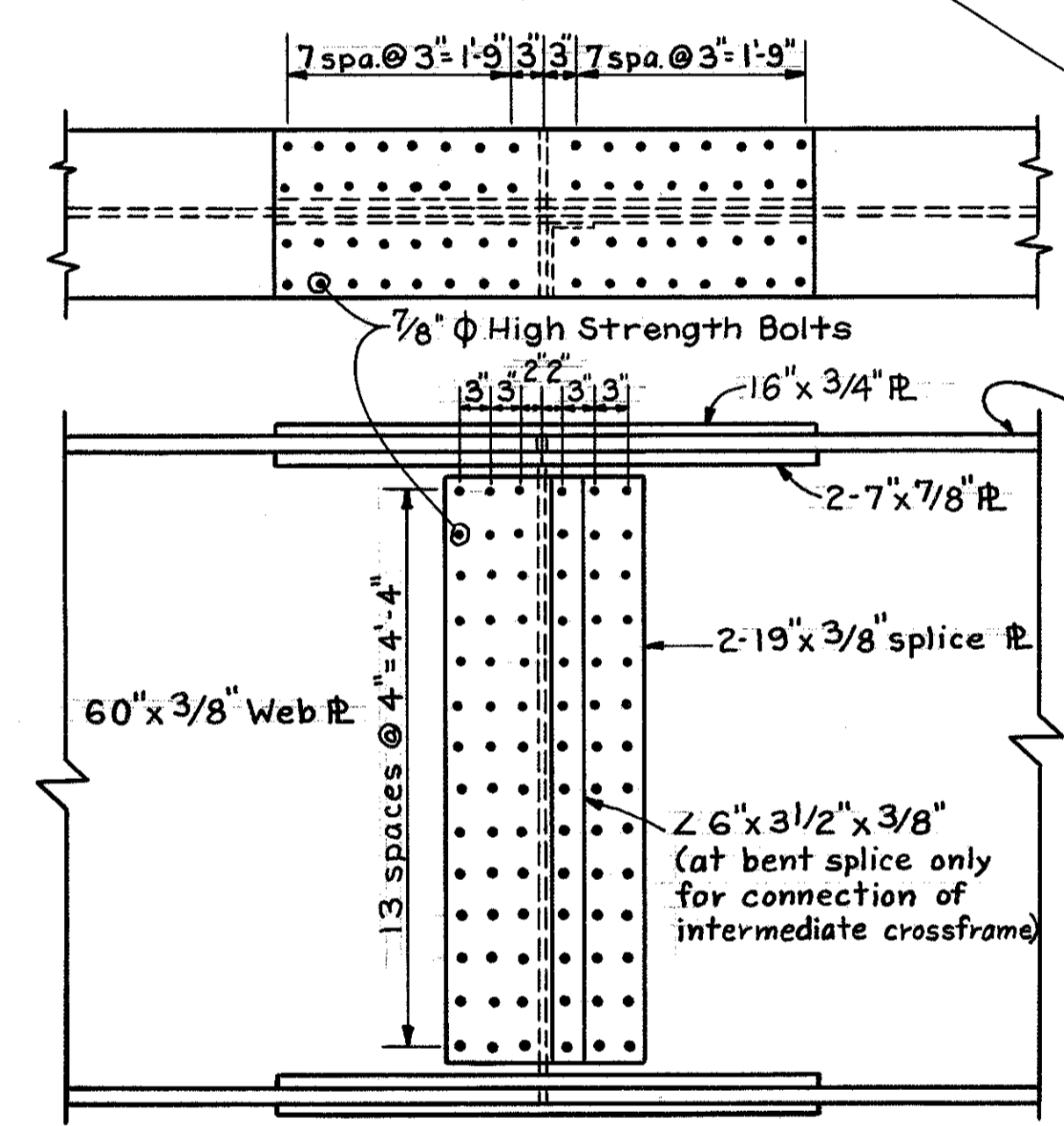
H&E BRIDGE No. 3

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	MJE	RS	JHO	8/31/64	

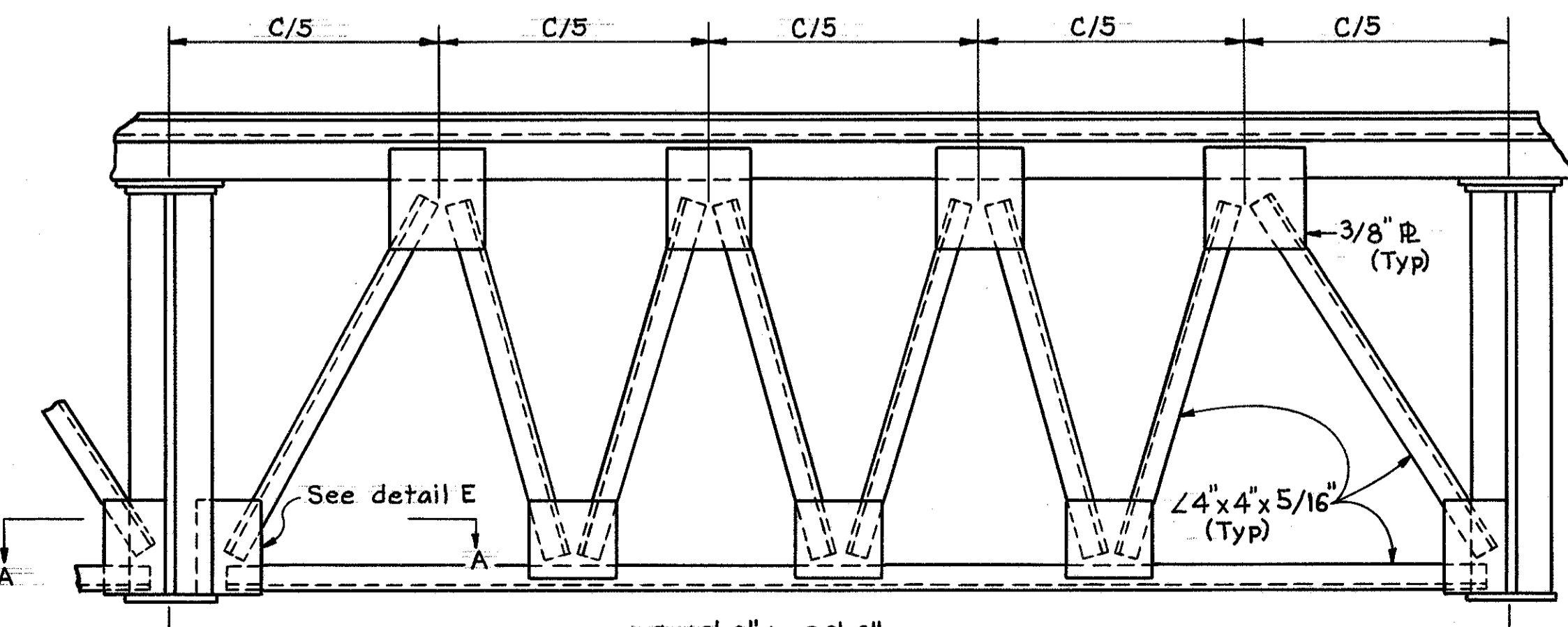
MICROFILMED
FEB 20 1985



DETAIL C

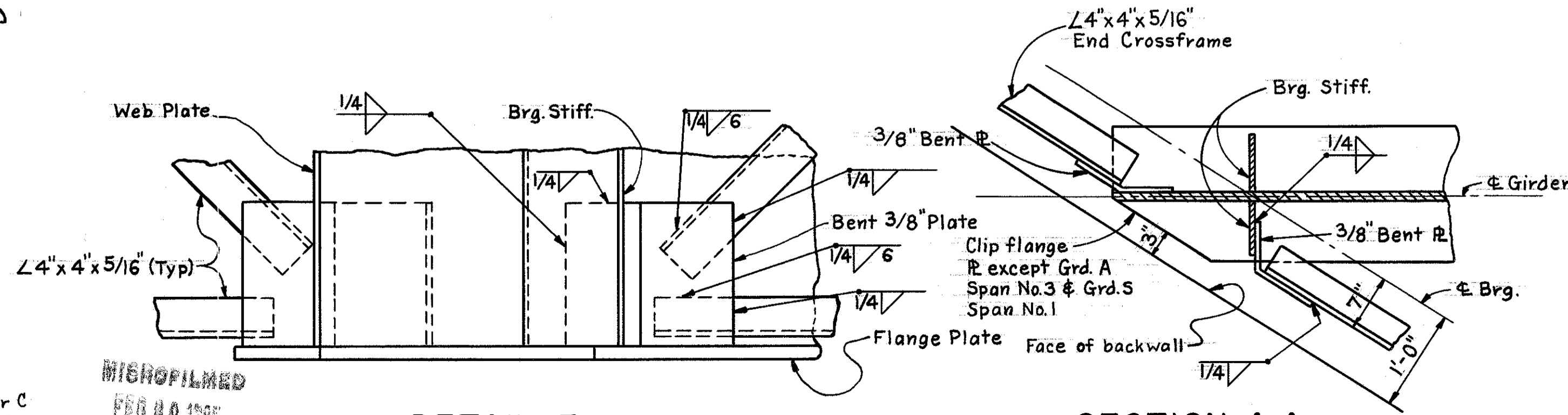


FIELD SPlice



END CROSSFRAME

Note: Unless otherwise shown all weld dimensions and member sizes for End Crossframes shall be as shown on Standard Drawing SD-1-63, Sh. No. 2



DETAIL E

SECTION A-A

Work this sheet with sheet Nos. 360 & 362

HAZEL & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

STRUCTURAL STEEL DETAILS

BRIDGE No. HAM.-71-0173

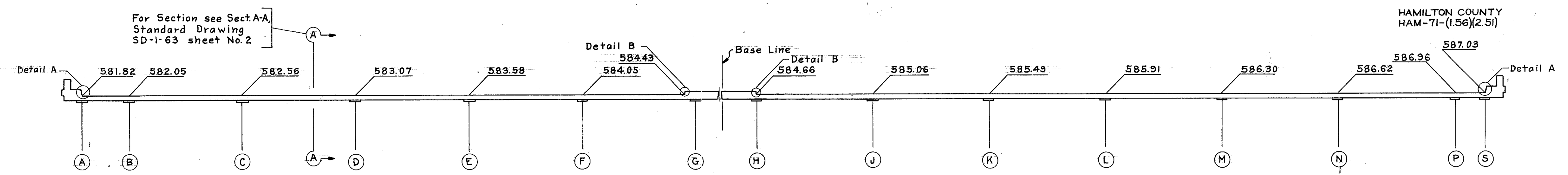
H&E BRIDGE No. 3

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	MJE	RS	JHO	8/31/64	
				5-20-65	

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

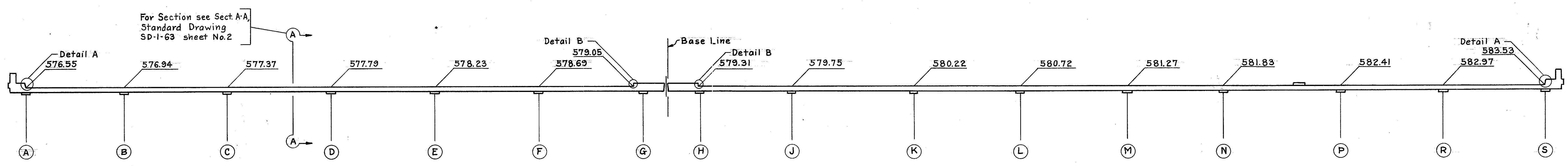
363
492

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

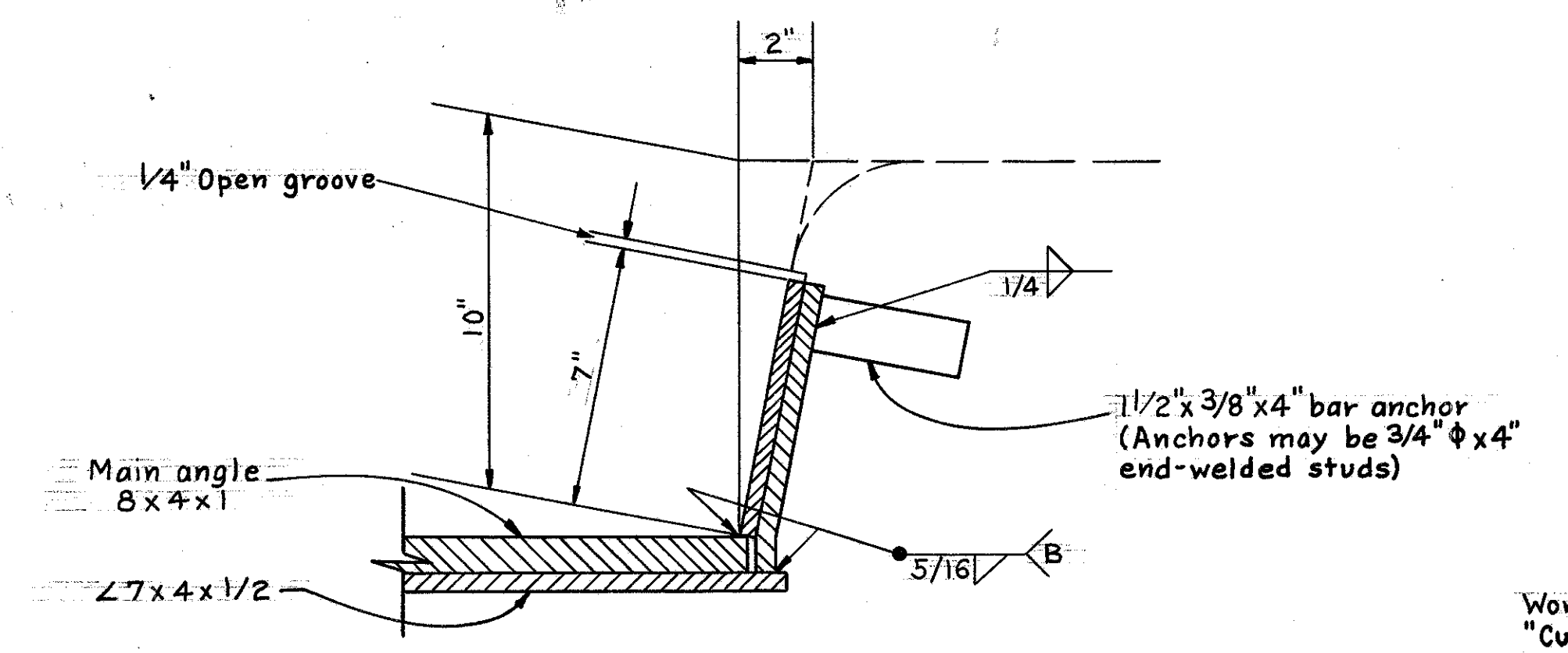


EXPANSION JOINT CROSS SECTION FOR NORTH ABUTMENT

Note:
Elevations given at
front face of backwall.
(Back of 7x4x1/2 L)

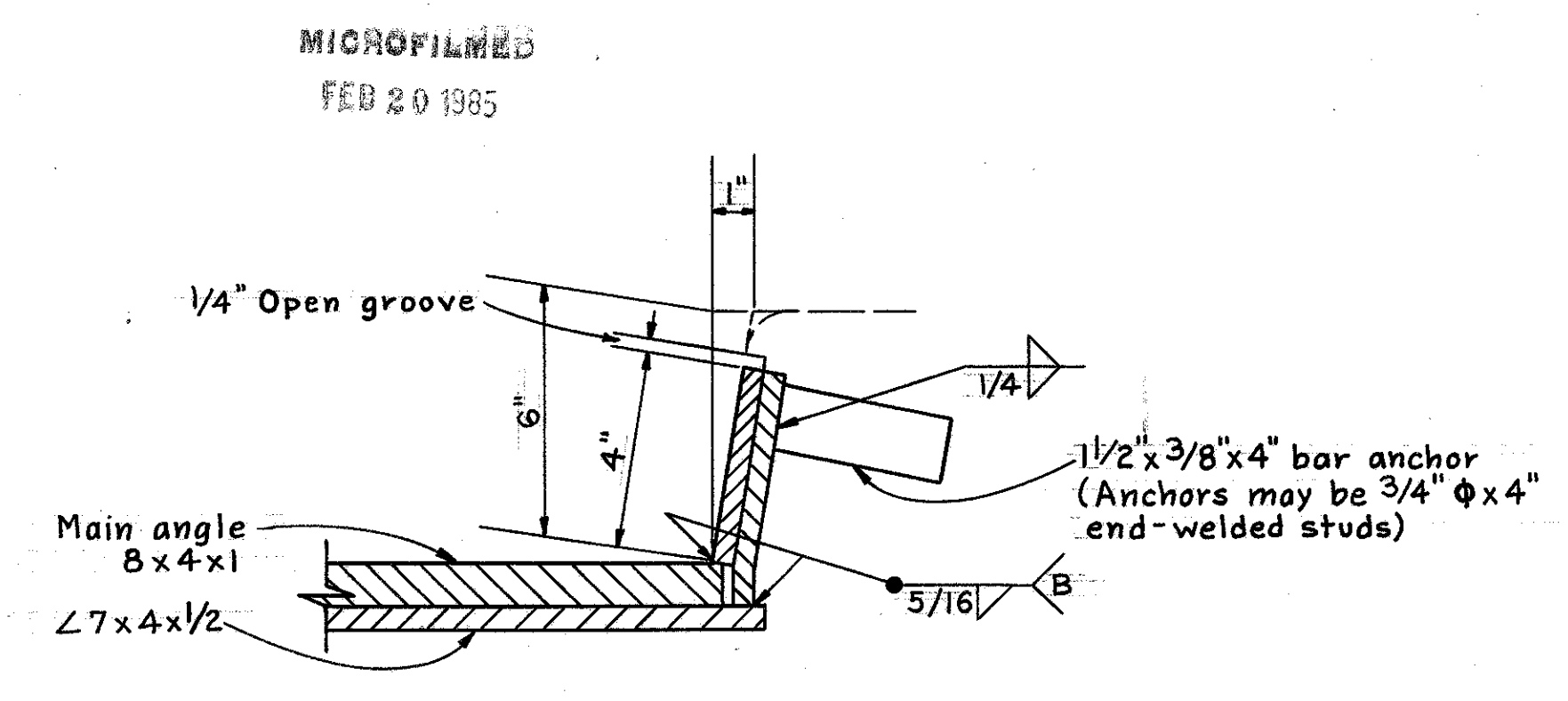


EXPANSION JOINT CROSS SECTION FOR SOUTH ABUTMENT



DETAIL A

Work Detail A & Detail B with
"Curb Plate Details" Standard
Drawing SD-1-63 sheet No. 4



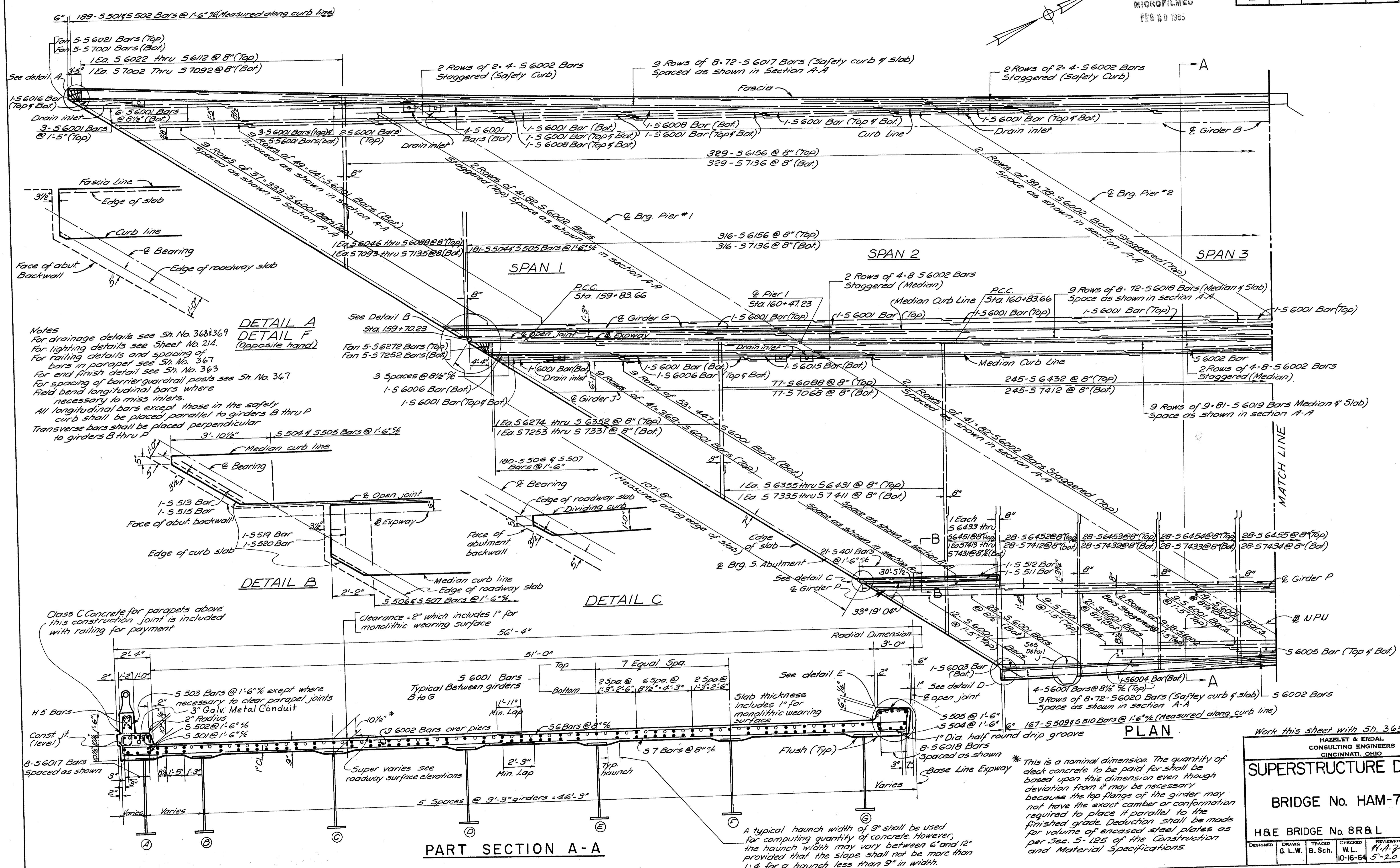
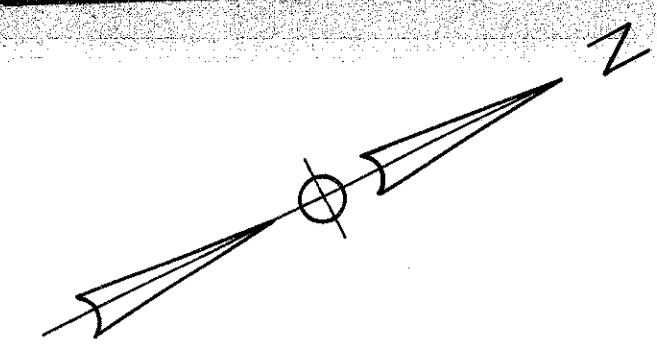
DETAIL B

MICROFILMED
FEB 20 1965

MICROFILMED
FEB 20 1965

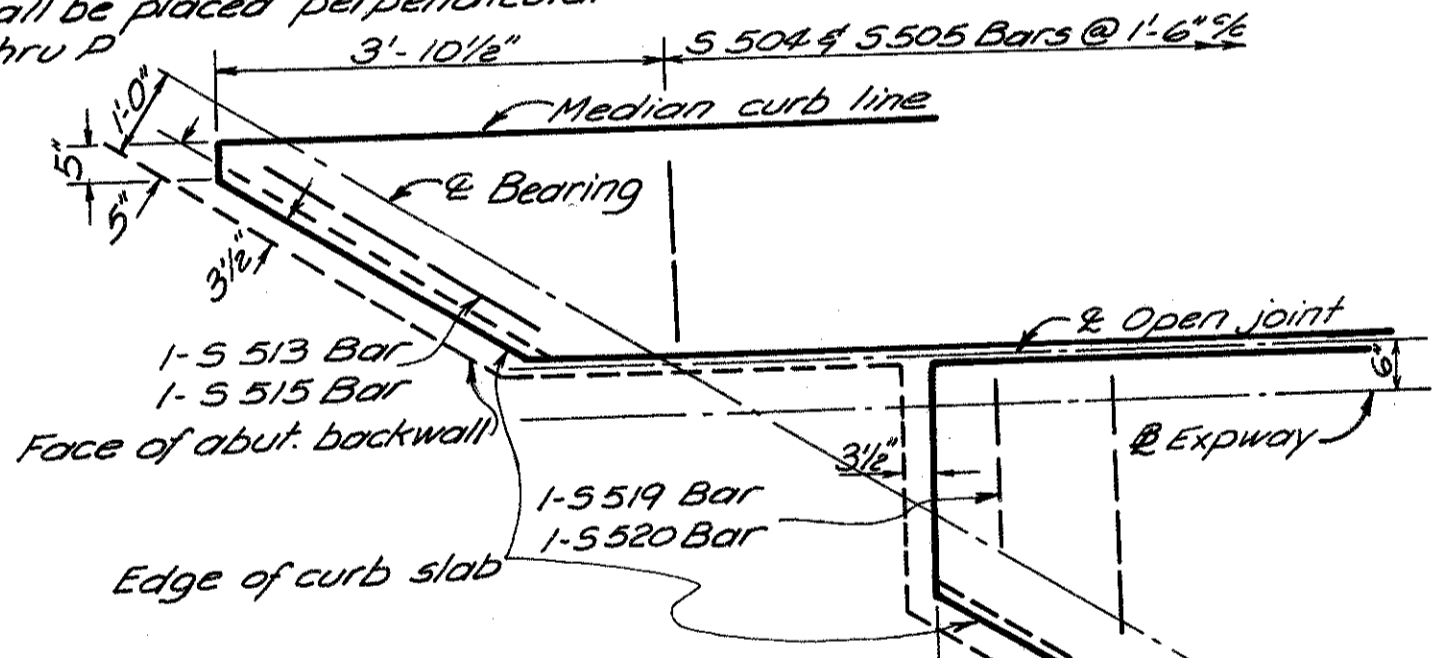
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
STRUCTURAL STEEL DETAILS					
BRIDGE No. HAM-71-0173					
H&E BRIDGE No. 8					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	MJE	RS	JHO	H&E 2/5/65	5-20-65

MICROFILMED
FEB 20 1985

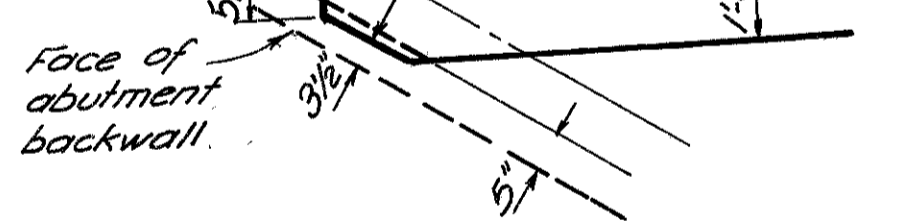


Notes
For drainage details see Sh. No. 368 & 369
For lighting details see Sheet No. 214.
For railing details and spacing of bars in parapet see Sh. No. 367
For end finish detail see Sh. No. 363
For spacing of barrier guardrail posts see Sh. No. 367
Field bend longitudinal bars where necessary to miss inlets.
All longitudinal bars except those in the safety curb shall be placed parallel to girders B thru P
Transverse bars shall be placed perpendicular to girders B thru P

DETAIL A
DETAIL F
(Opposite hand)

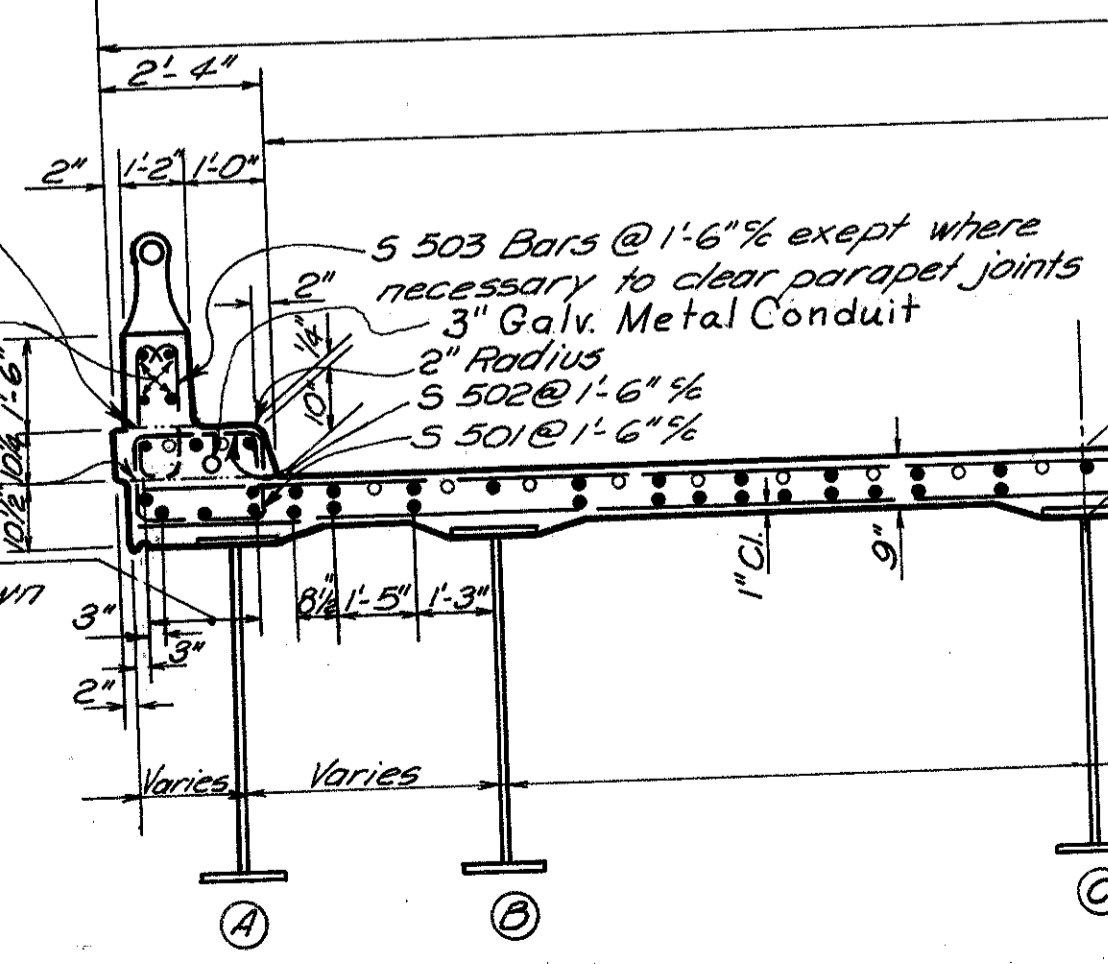


DETAIL B

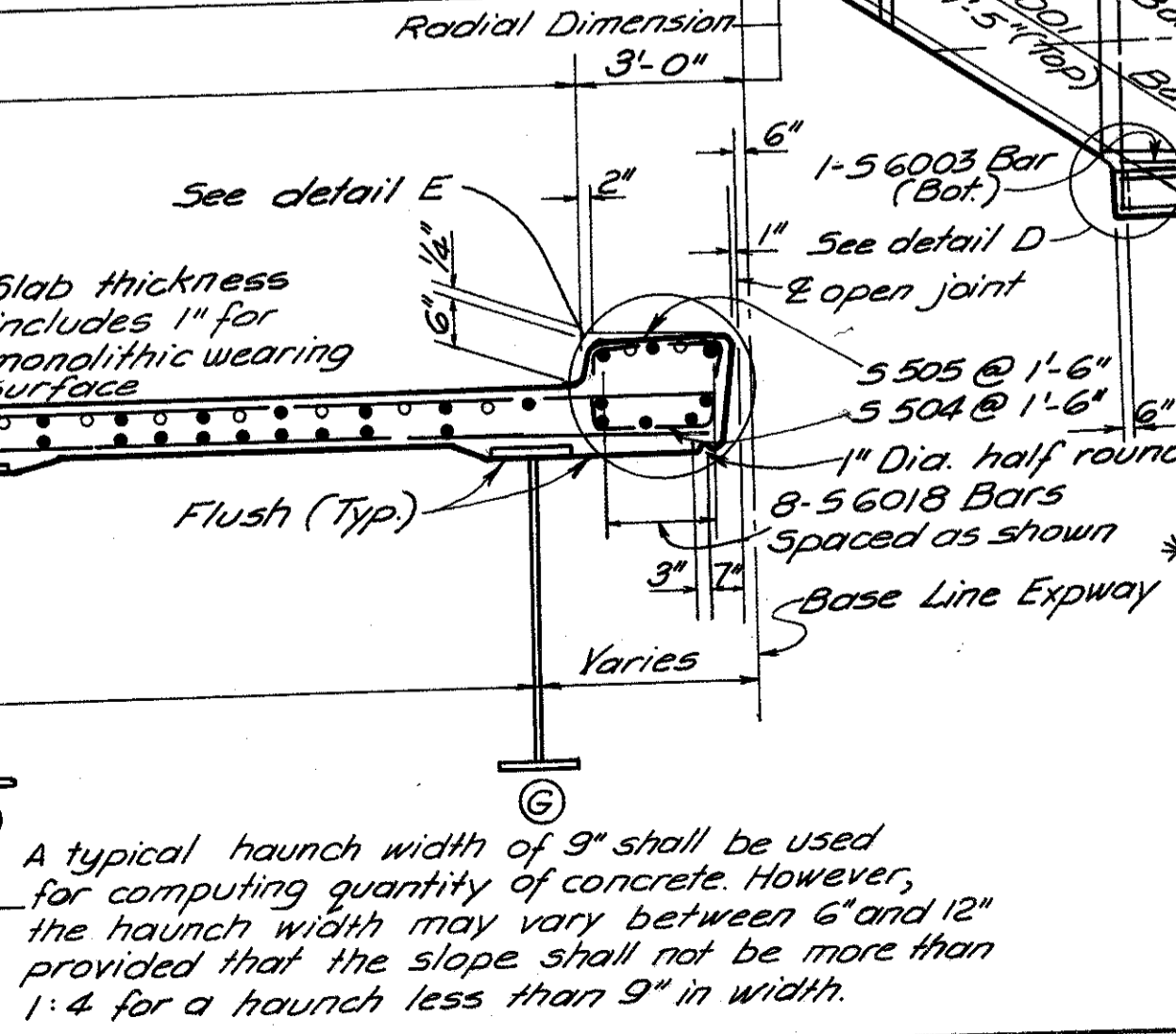


DETAIL C

Class C Concrete for parapets above this construction joint is included with railing for payment



PART SECTION A-A



Slab thickness includes 1\"/>

A typical haunch width of 9\"/>

This is a nominal dimension. The quantity of deck concrete to be paid for shall be based upon this dimension even though deviation from it may be necessary because the top flange of the girder may not have the exact camber or conformation required to place it parallel to the finished grade. Deduction shall be made for volume of encased steel plates as per Sec. 5-125 of the Construction and Material Specifications.

Work this sheet with Sh. 365

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SUPERSTRUCTURE DETAILS

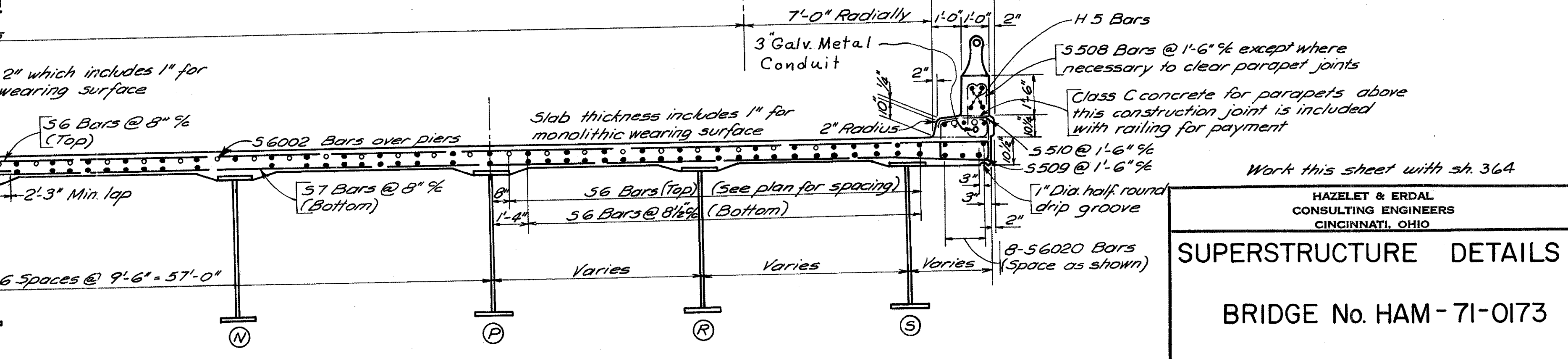
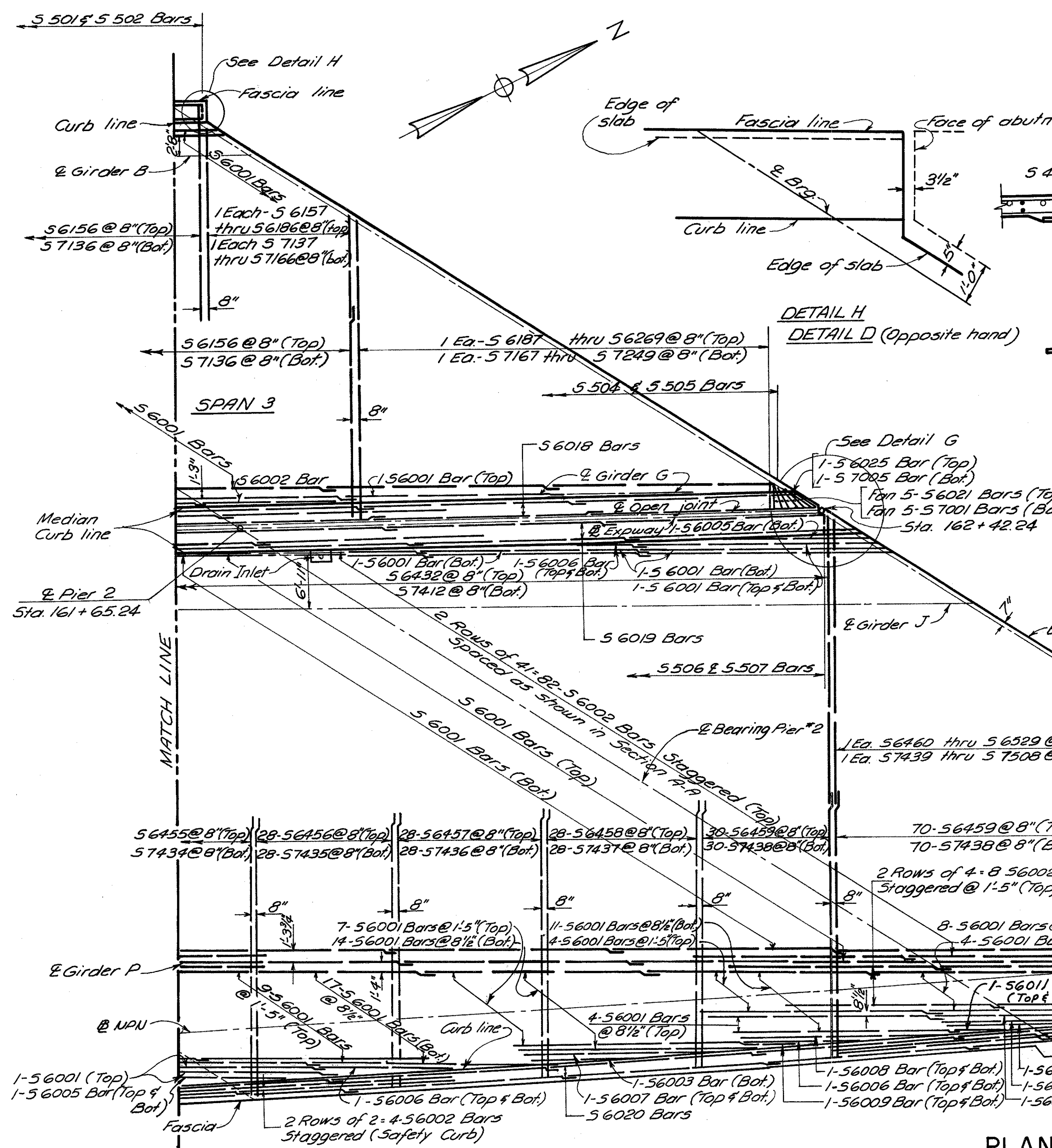
BRIDGE No. HAM-71-0173

H&E BRIDGE No. 8R&L

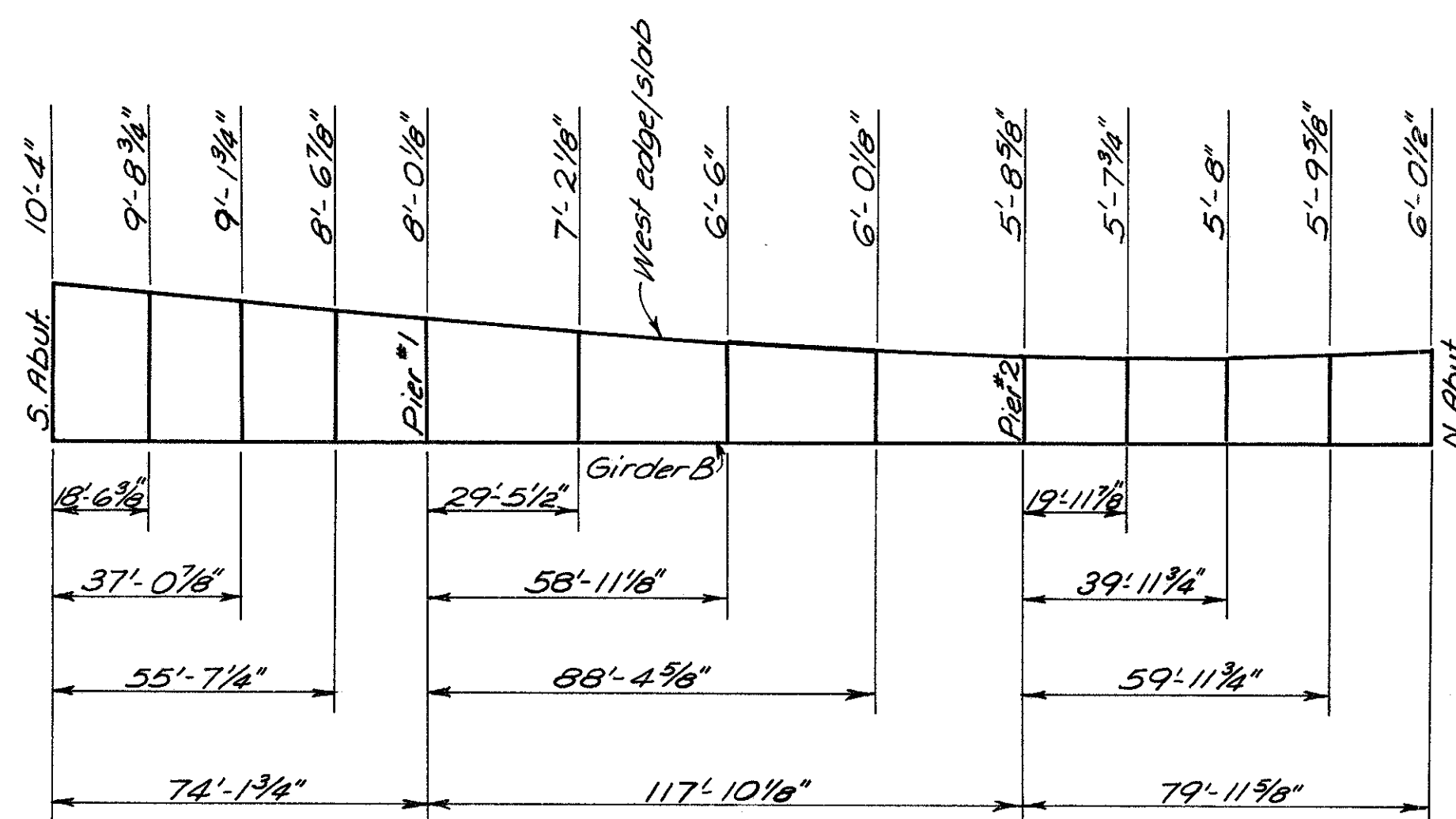
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
G. L. W.	B. Sch.	W. L.	H. A. Z.	10-16-64	5-20-65

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

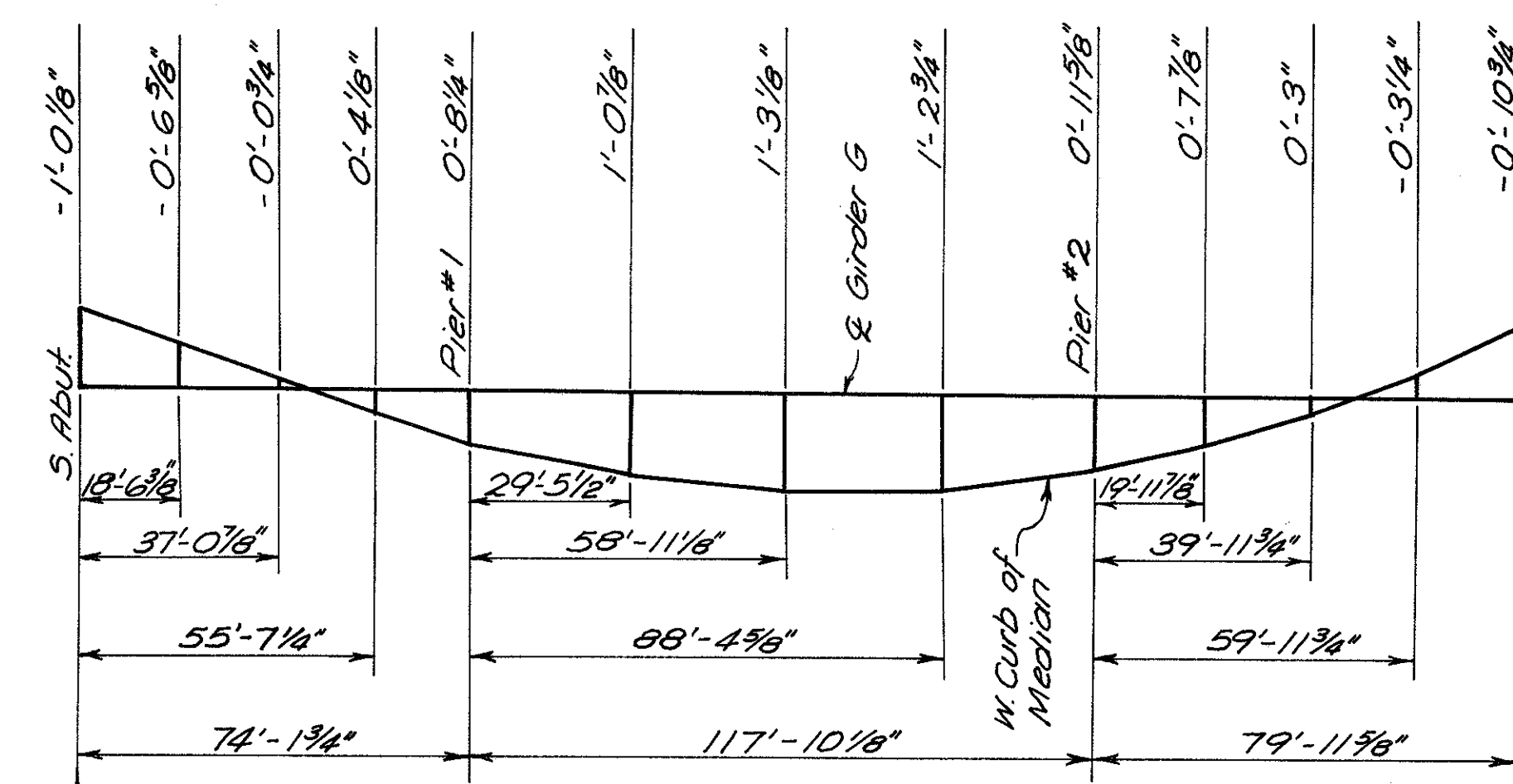
For detail of Guard Rail
see standard drawing
I-15 No. 2-A



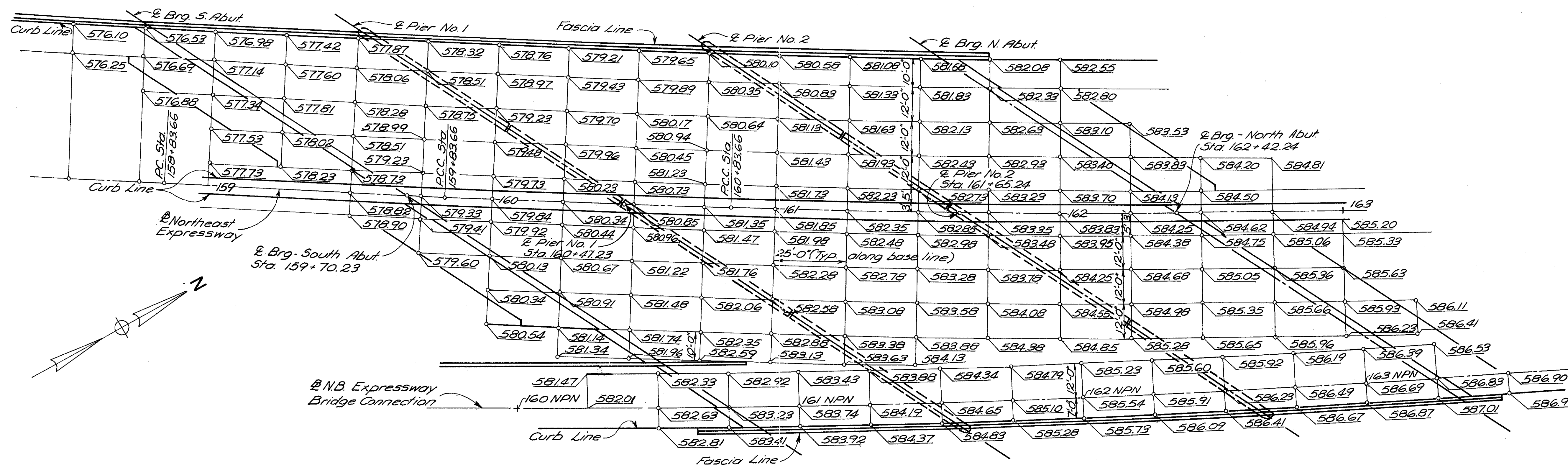
PART SECTION A-A



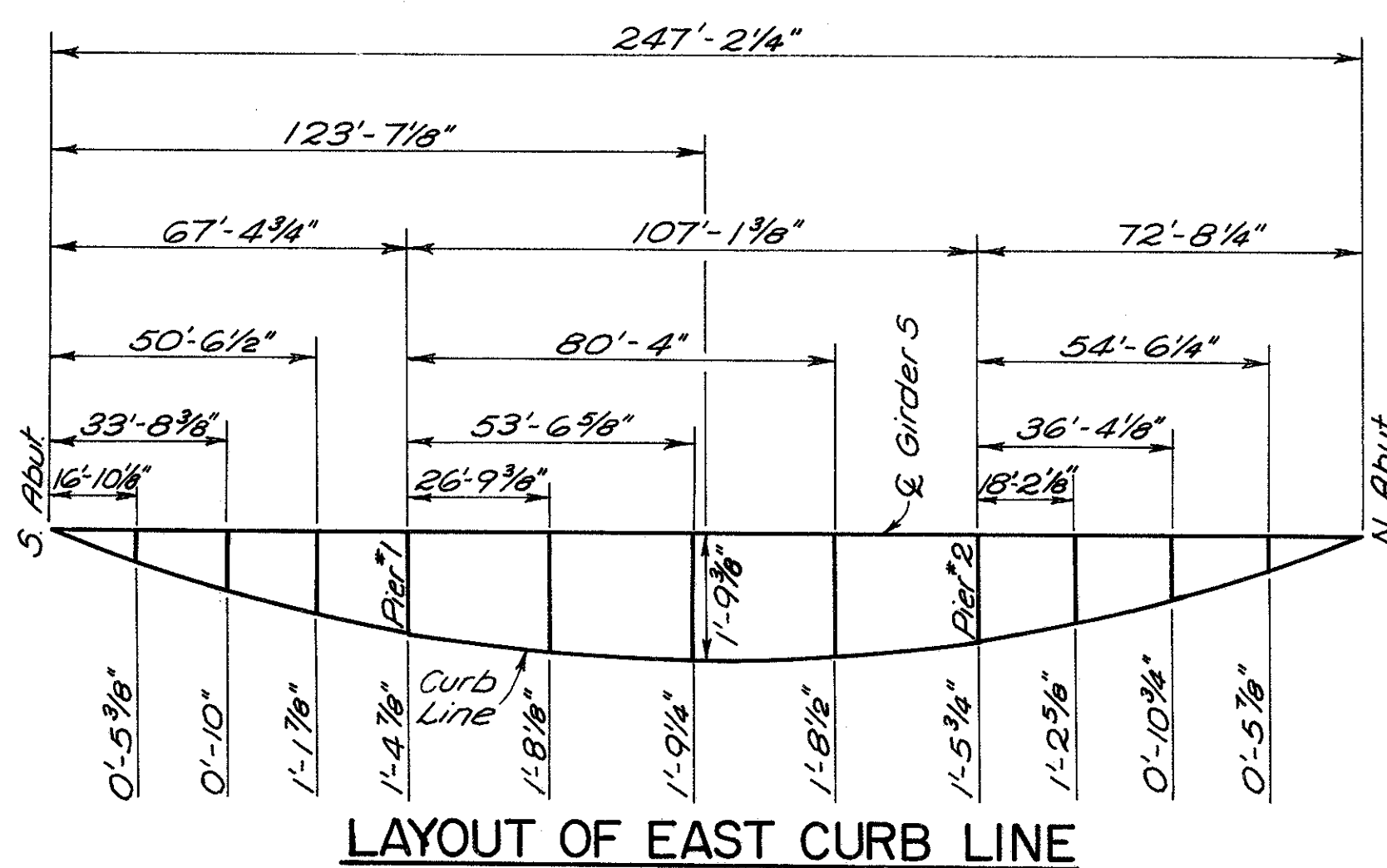
LAYOUT OF WEST EDGE OF ROADWAY SLAB



LAYOUT OF WEST CURB OF MEDIAN



ROADWAY SURFACE ELEVATIONS



LAYOUT OF EAST CURB LINE

MICROFILMED
FEB 20 1985

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SUPERSTRUCTURE DETAILS

BRIDGE NO. HAM.-71-0173

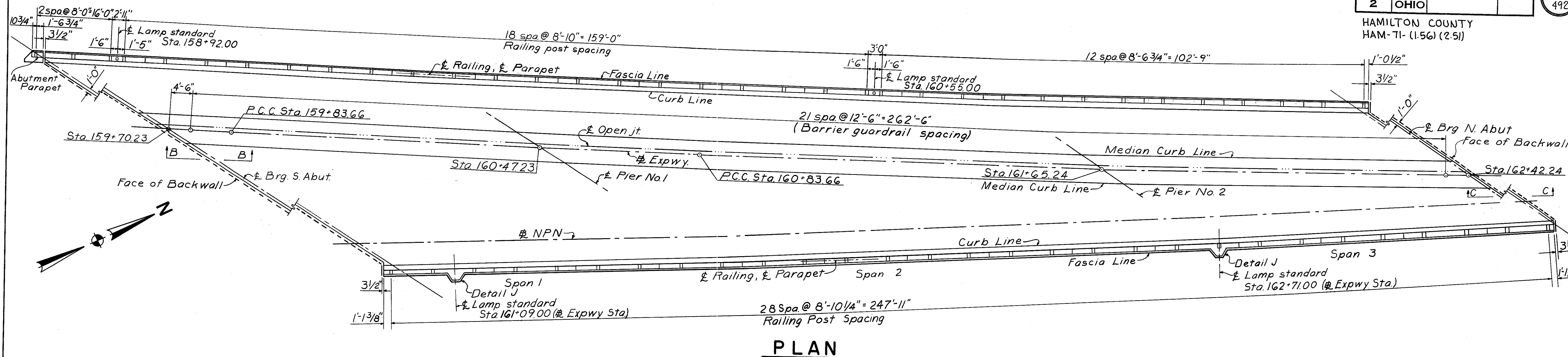
H & E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
M. J. E.	B. Sch.	J. H. O.		M. H. J. 5-26-63	

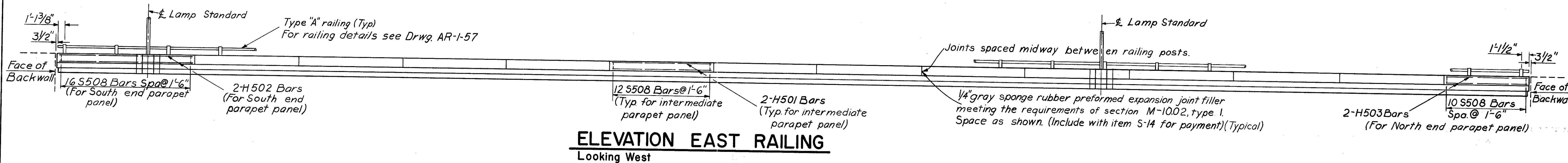
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

367
492

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

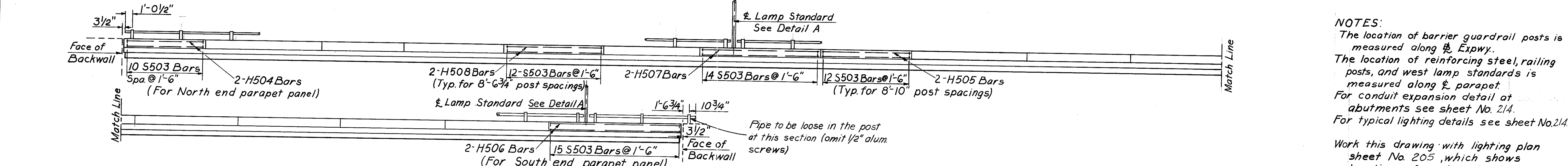


PLAN



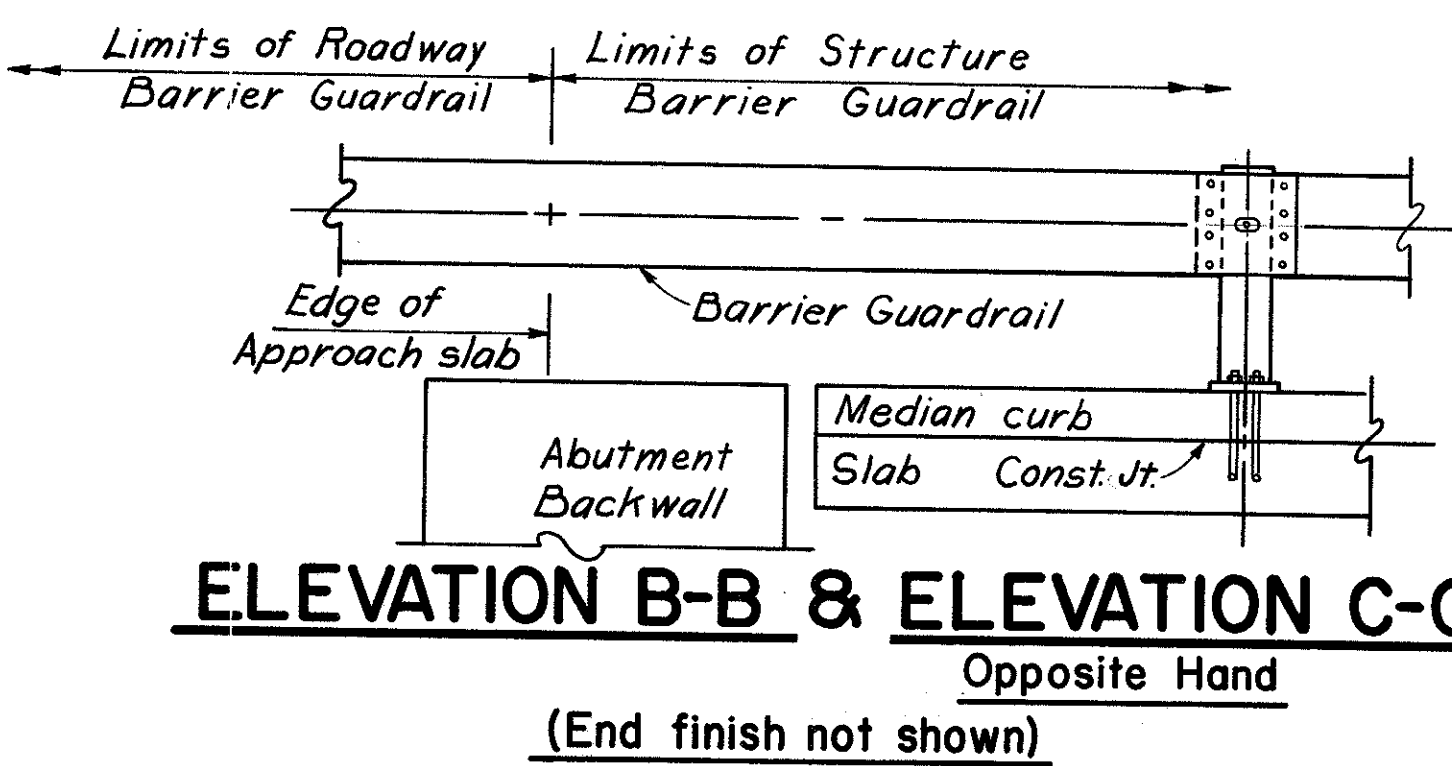
ELEVATION EAST RAILING

Looking West



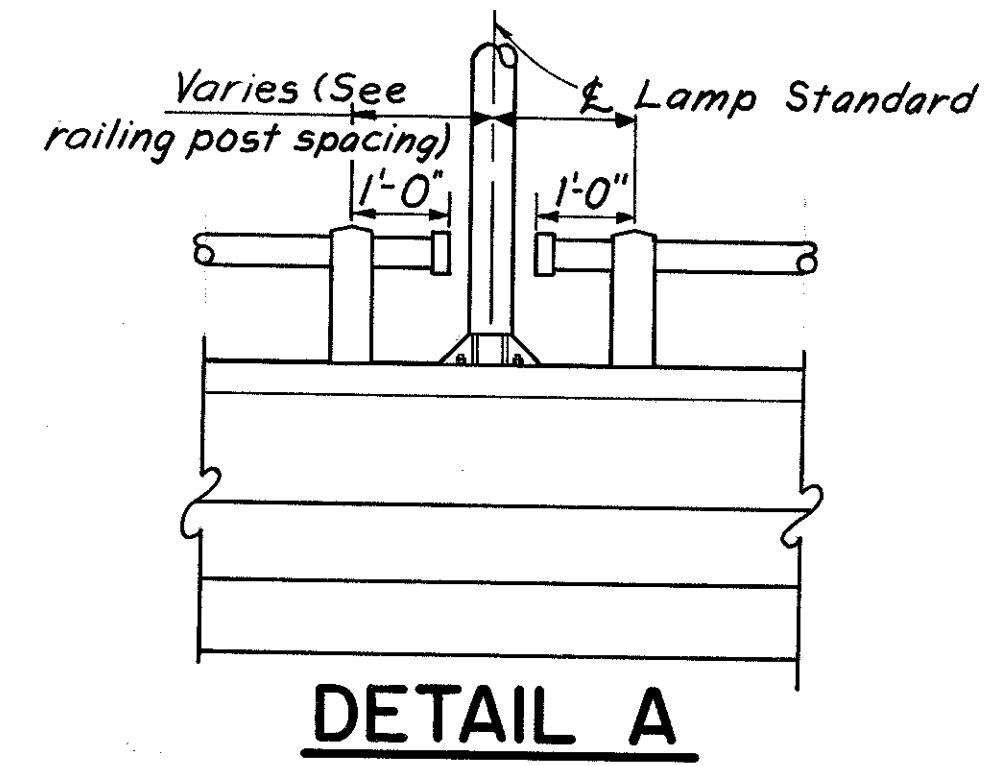
ELEVATION WEST RAILING

Looking East



ELEVATION B-B & ELEVATION C-C

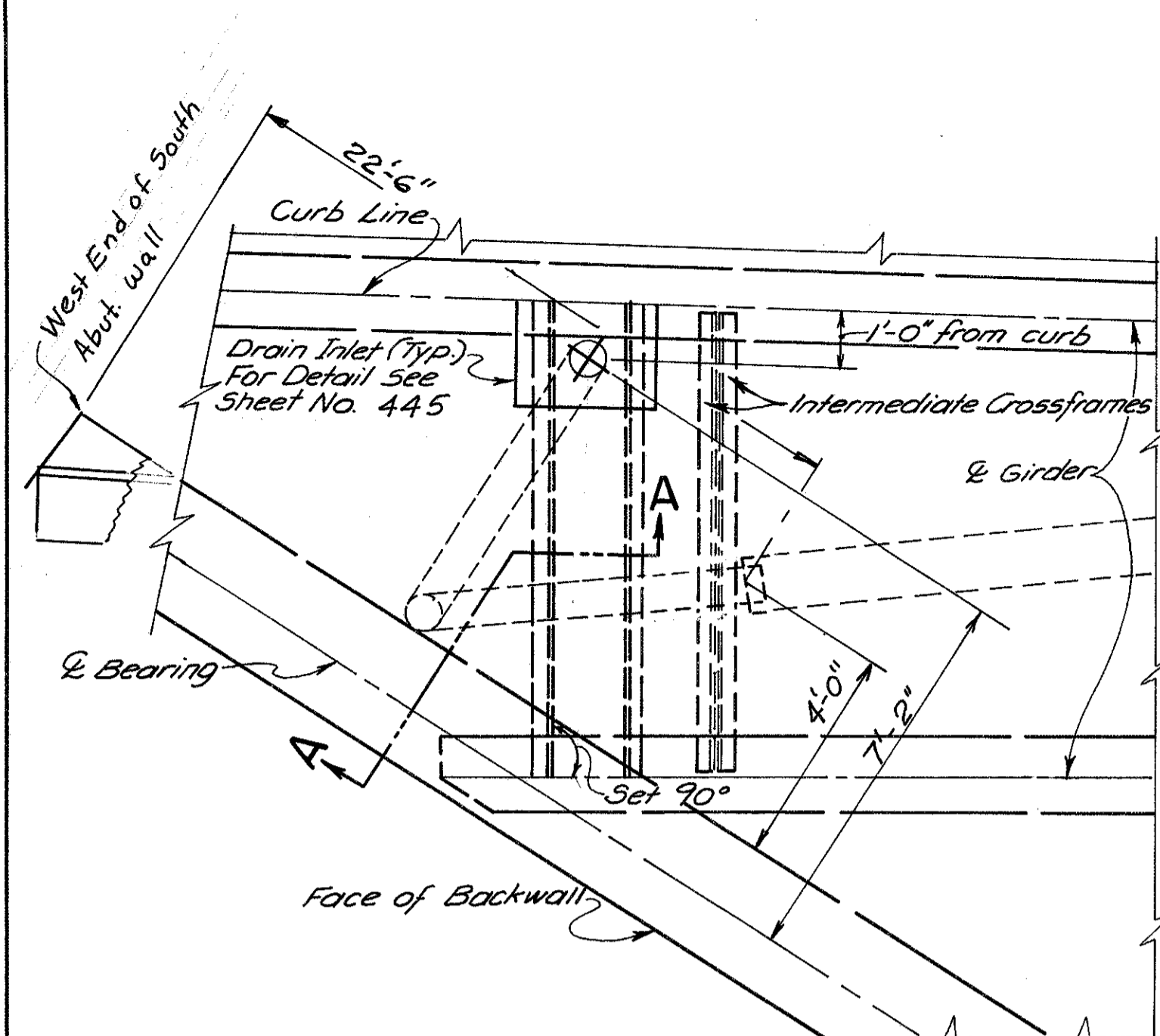
Opposite Hand
(End finish not shown)



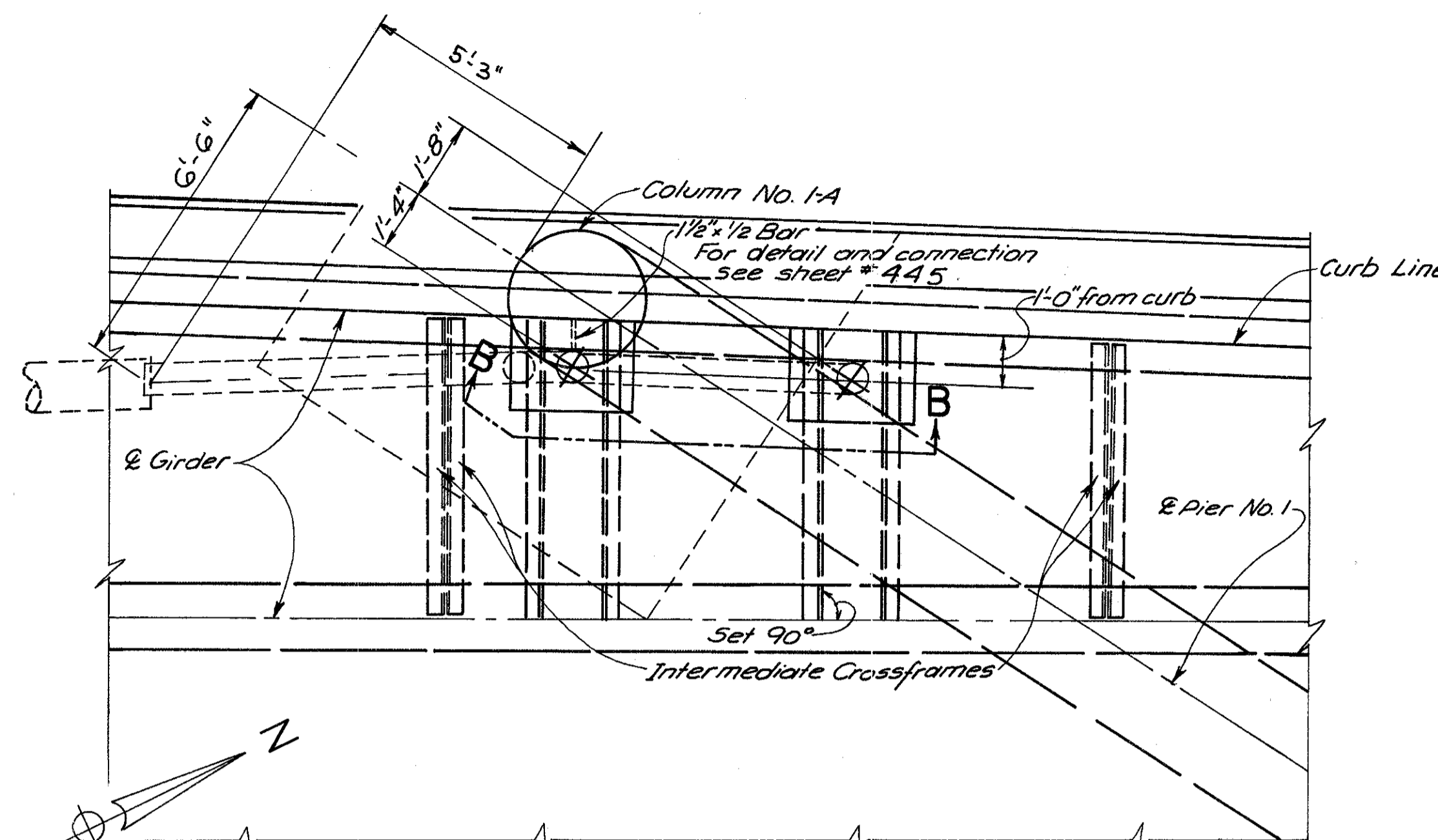
DETAIL A

NOTES:
The location of barrier guardrail posts is measured along the Expwy.
The location of reinforcing steel, railing posts, and west lamp standards is measured along the parapet.
For conduit expansion detail at abutments see sheet No. 214.
For typical lighting details see sheet No. 214.
Work this drawing with lighting plan sheet No. 205, which shows locations of conduits, hand holds, and lamp standards.
For detail J see sheet No. 365
For details of barrier guardrail see sheet No. 365

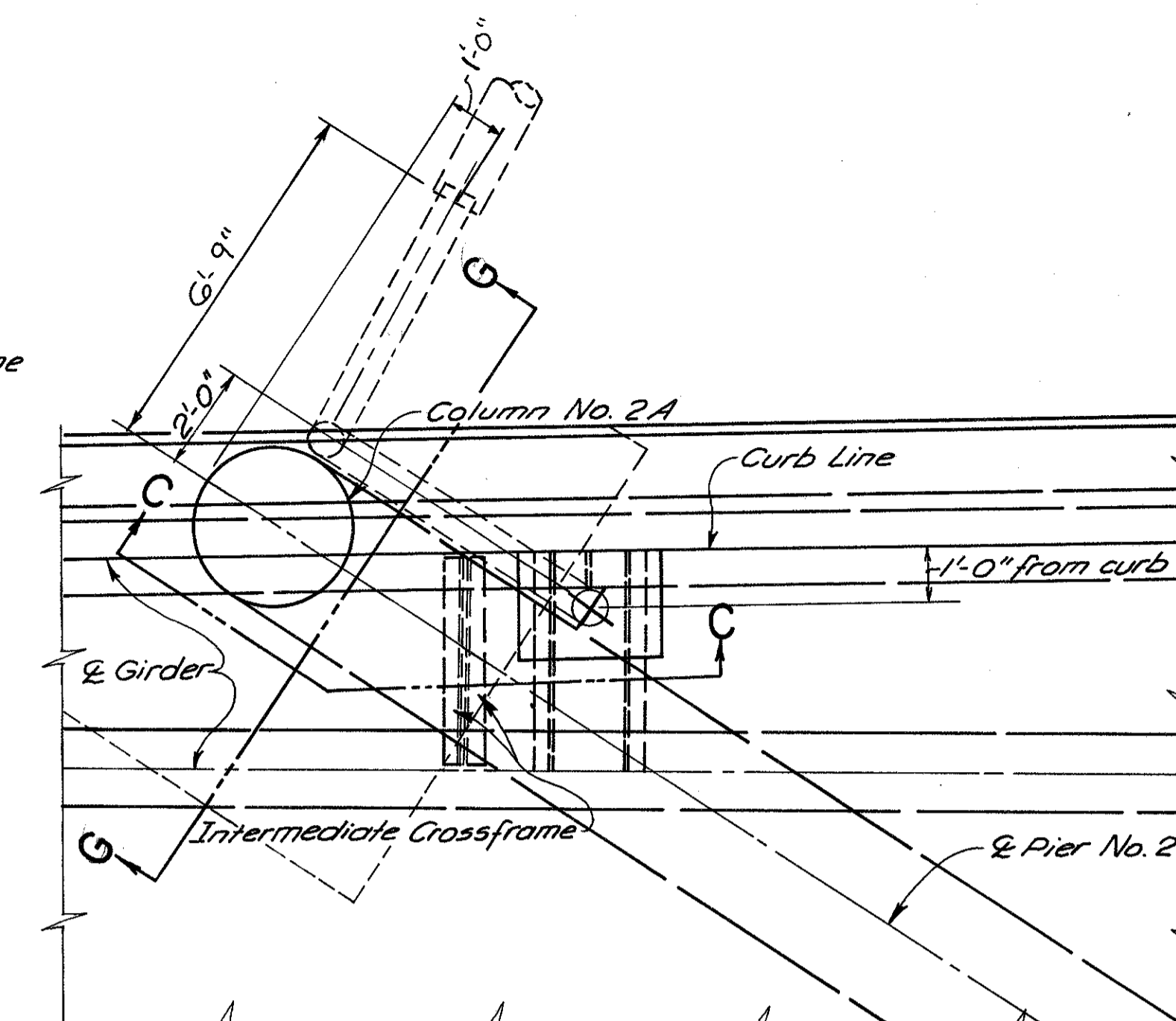
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
RAILING & LIGHTING					
BRIDGE No. HAM-71-0173					
H&E BRIDGE No. 8R&L					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
MJE	WL			5-20-65	



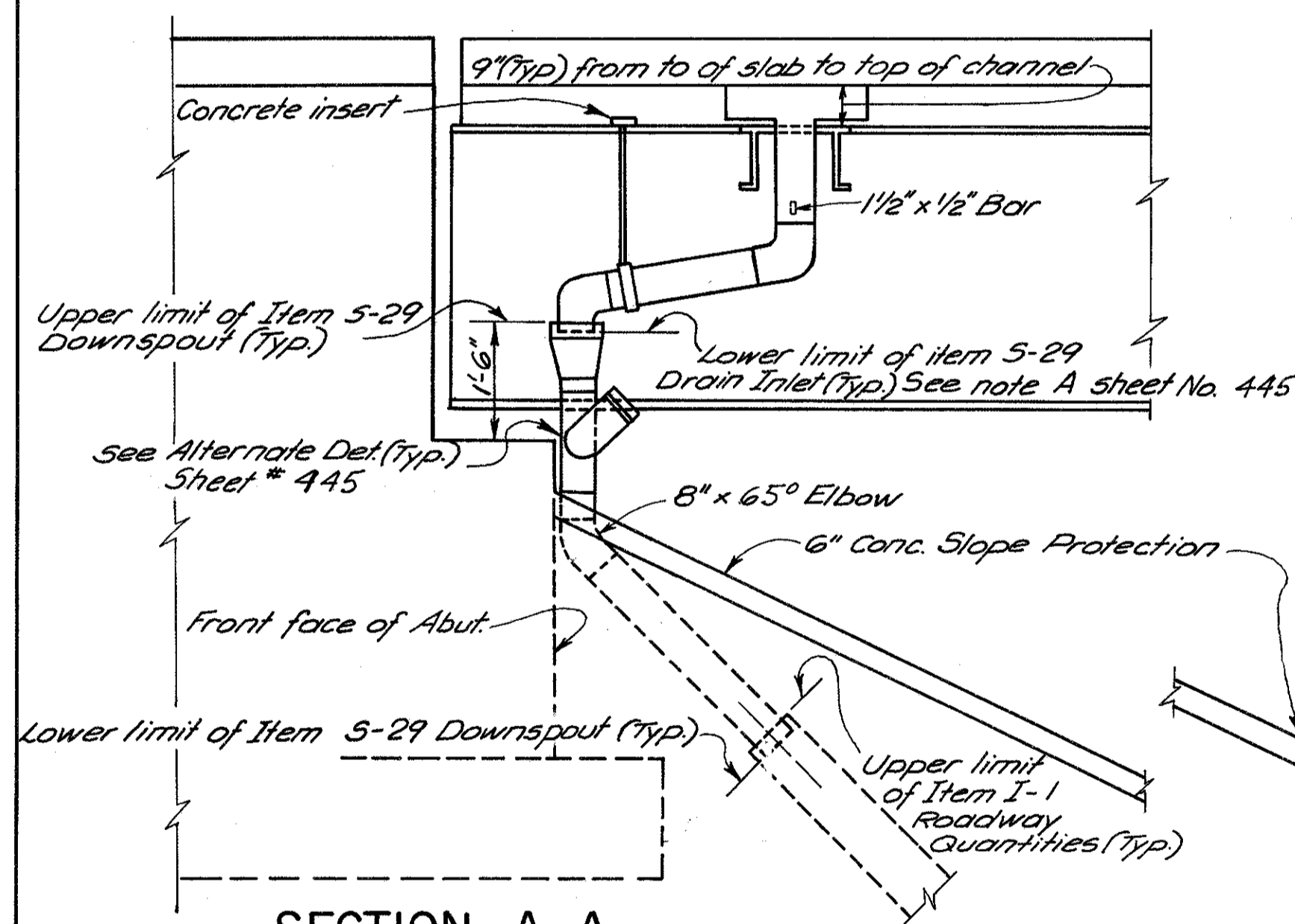
PLAN FOR DRAIN INLET AT S. ABUT. OF SOUTH BOUND ROADWAY



PLAN FOR DRAIN INLET AT PIER No. 1 OF SOUTH BOUND ROADWAY

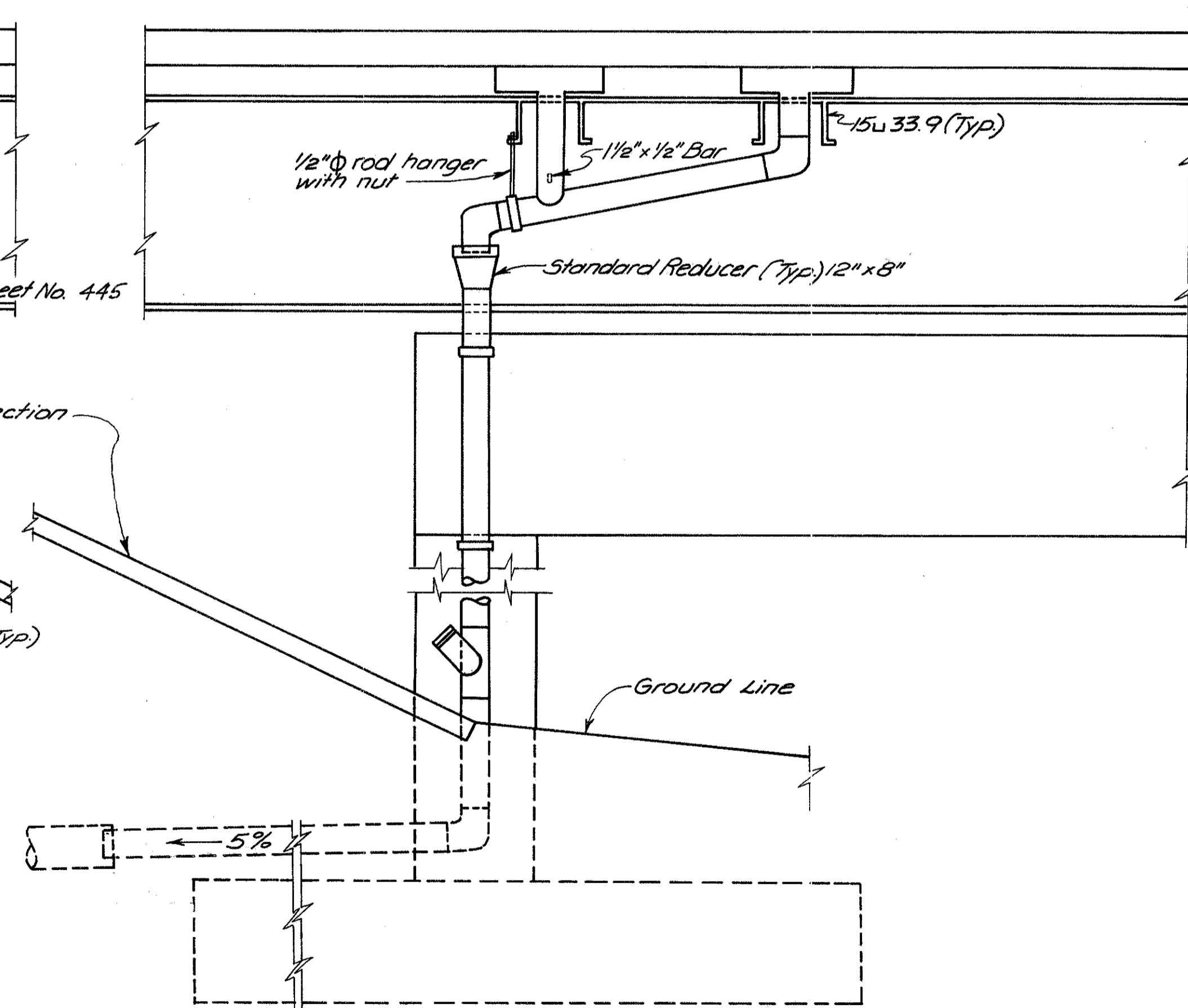


PLAN FOR DRAIN INLET AT PIER No. 2 OF SOUTH BOUND ROADWAY



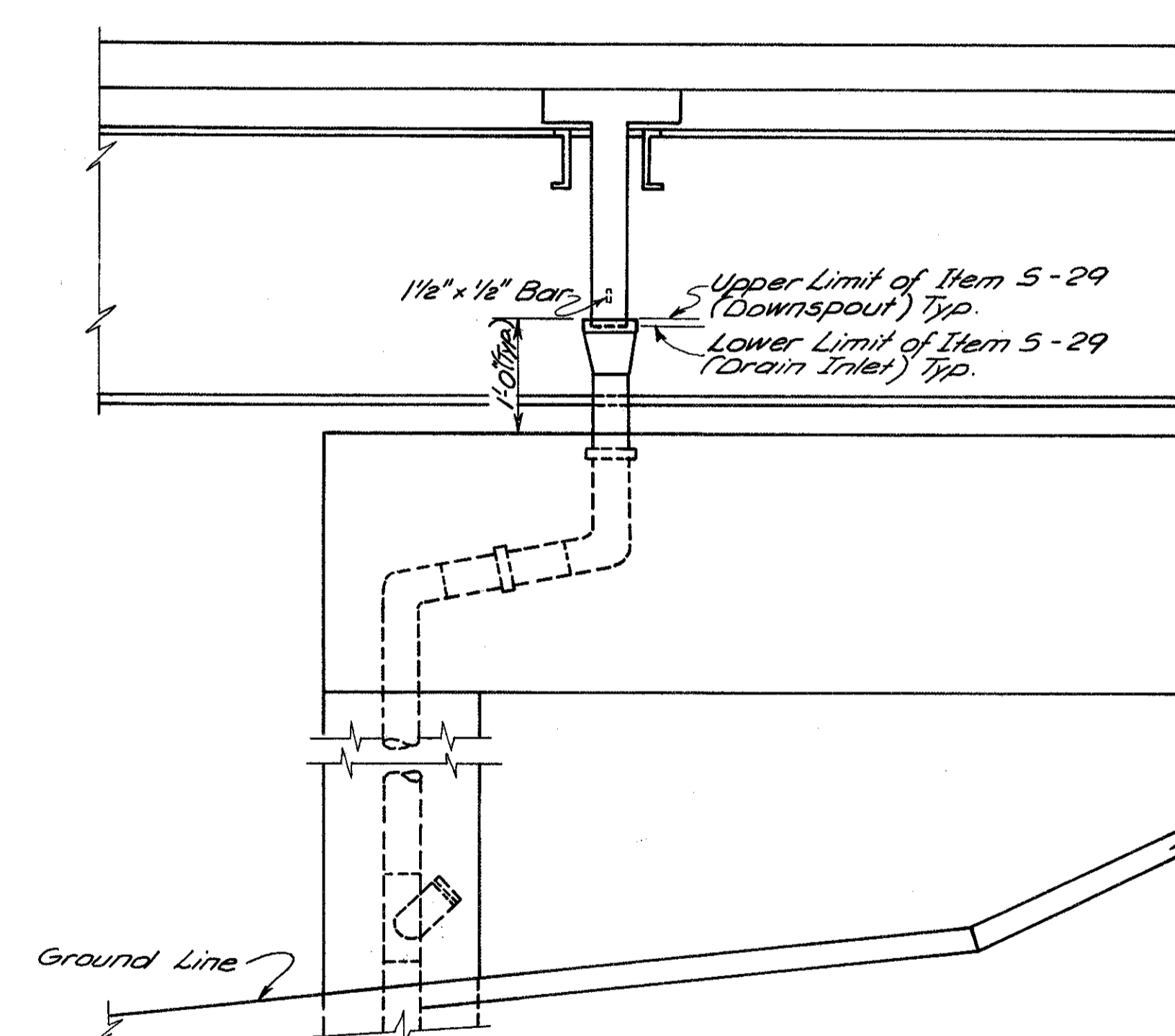
SECTION A-A

Note: Total depth of inlet frame box at S. Abut., Pier No. 1 and Pier No. 2 equals 7 1/4"

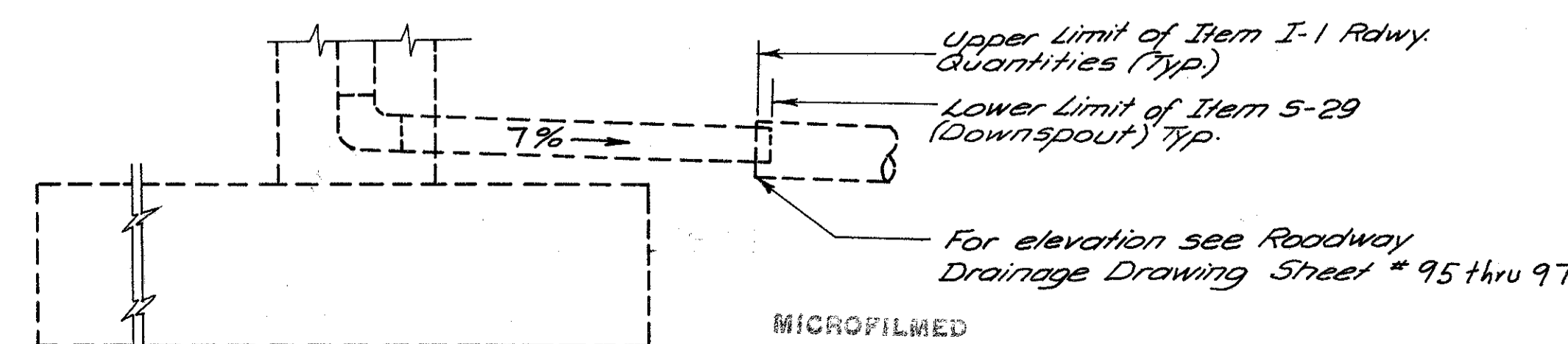


SECTION B-B

8" Pipe Throughout
Work this drawing with typical drawing sheet # 445



SECTION C-C



SECTION G-G

MICROFILMED
FEB 20 1985

Work this sheet with sheet No 369

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

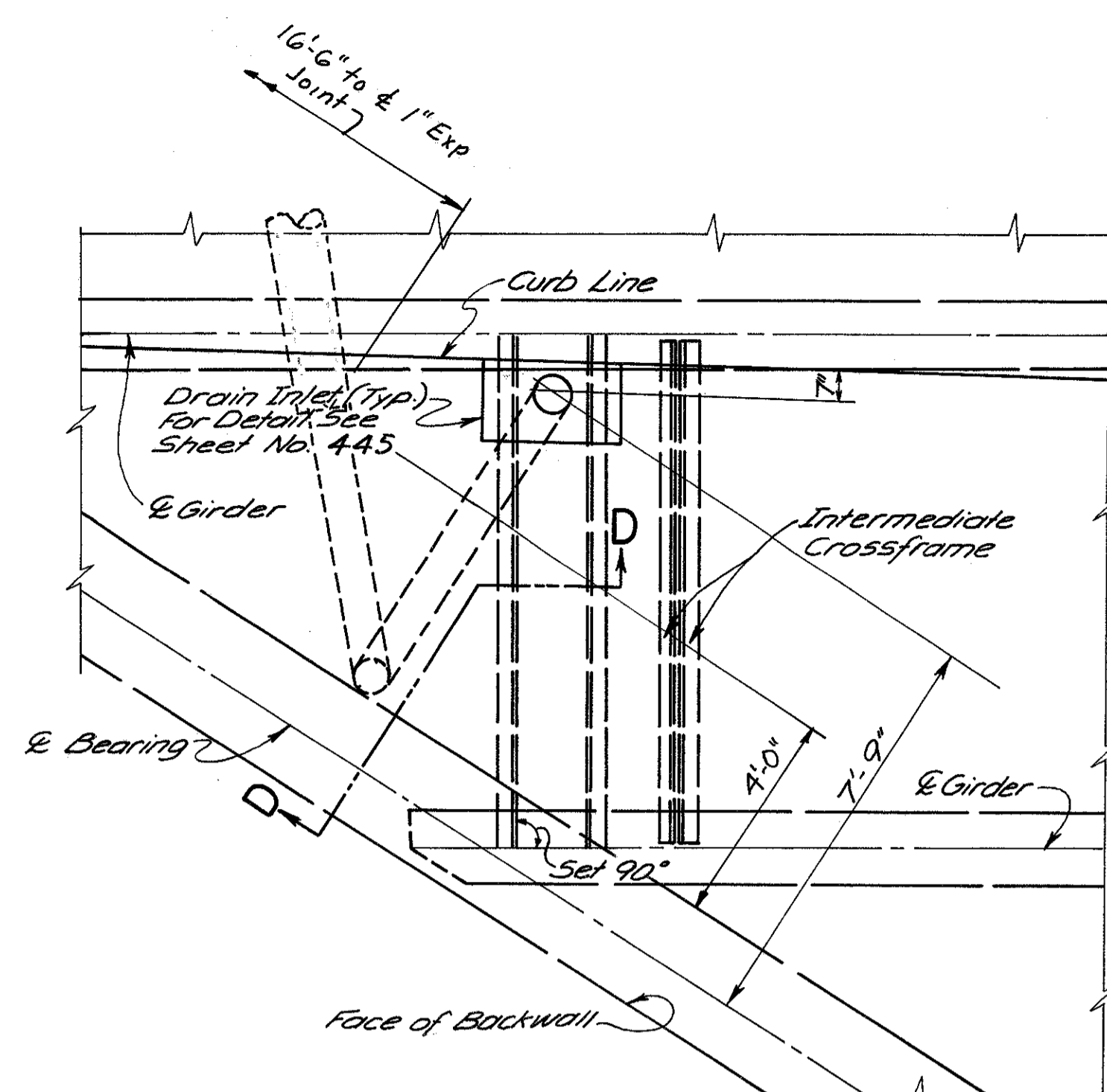
DRAINAGE DETAILS
BRIDGE No. HAM.-71-0173

H & E BRIDGE No. 8				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	M. E.	B. Sch.	J.H.O.	H.A.B.
			2-15-65	5-20-65

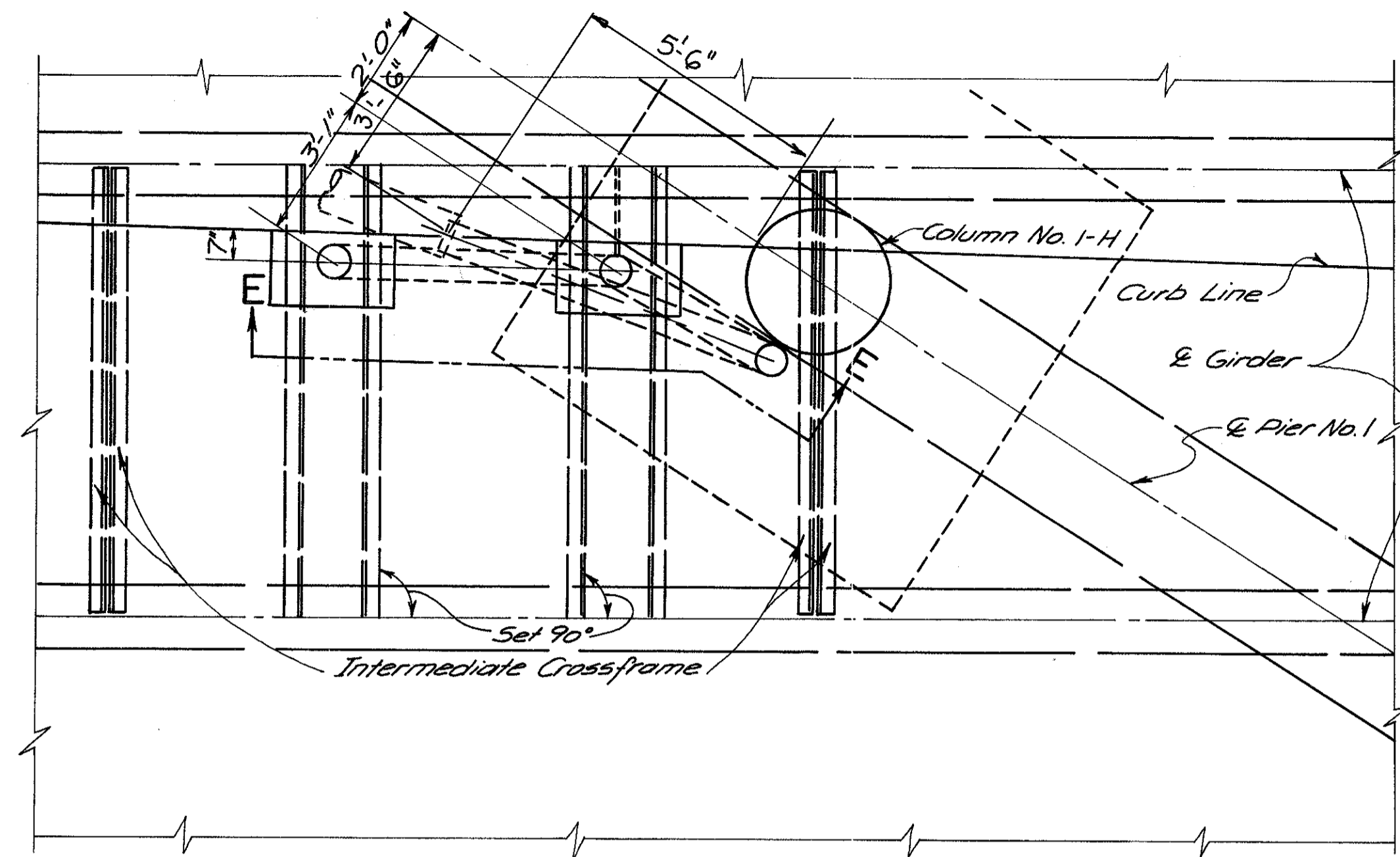
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

369
492

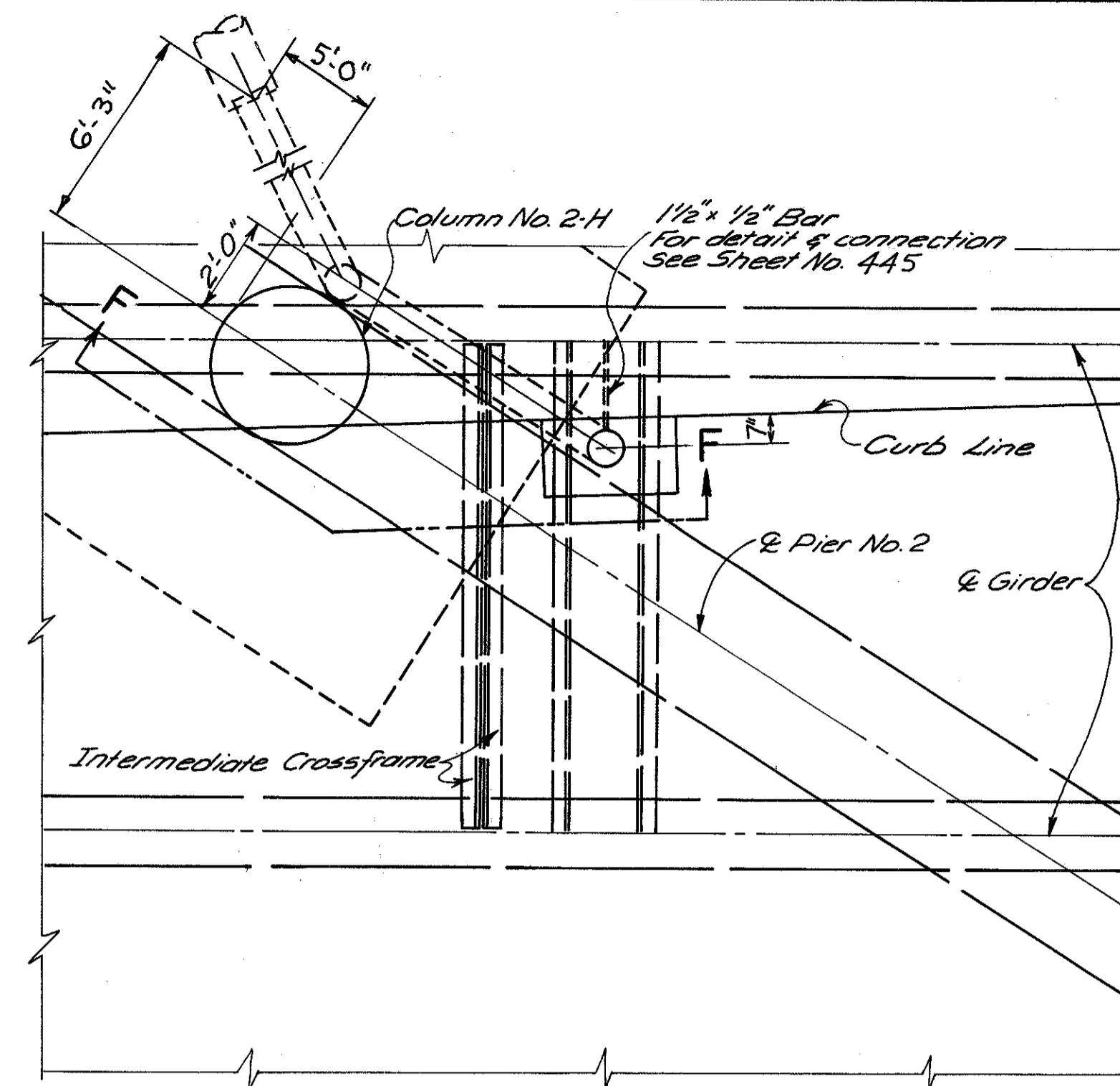
HAMILTON COUNTY
HAM-71- (1.56) (2.51)



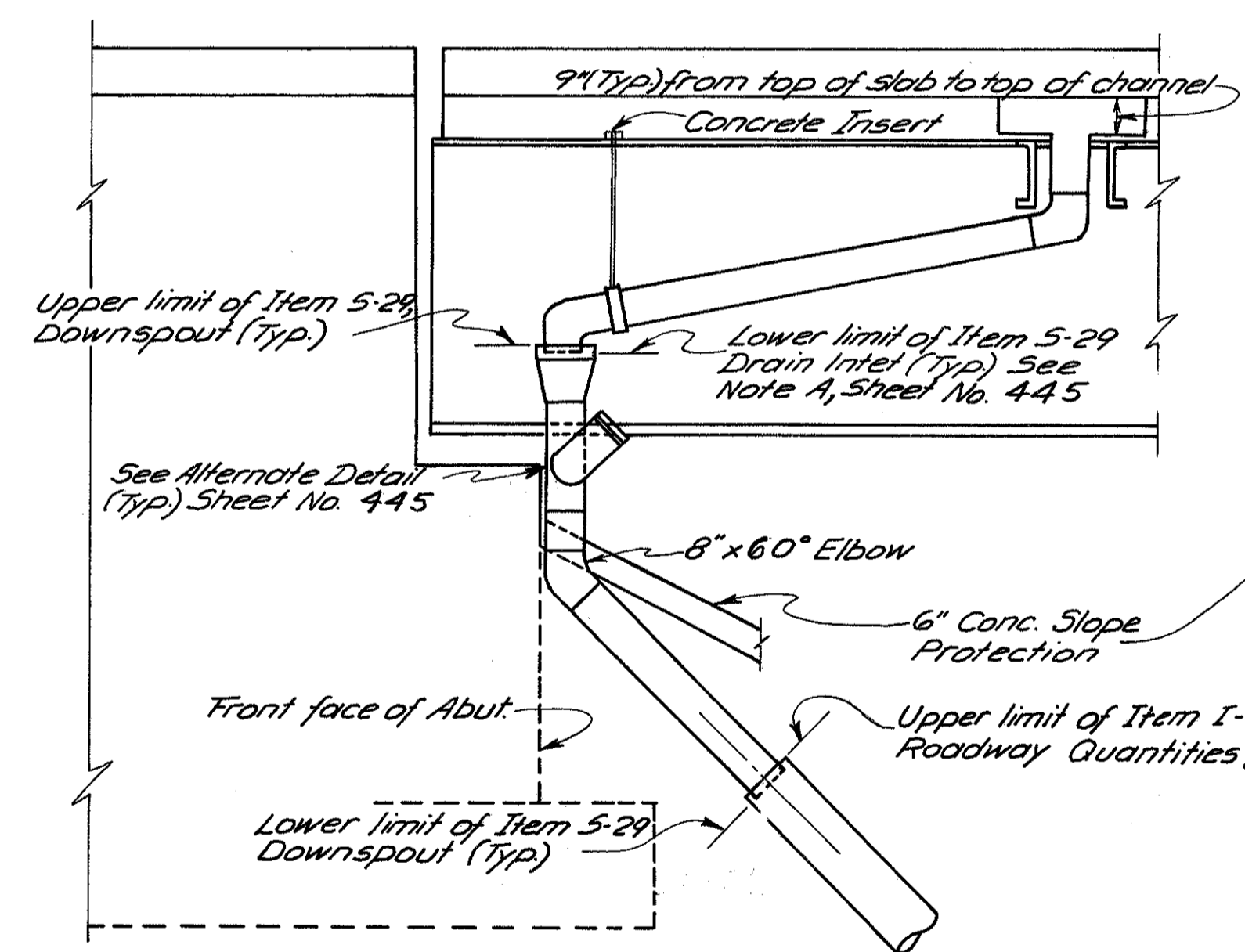
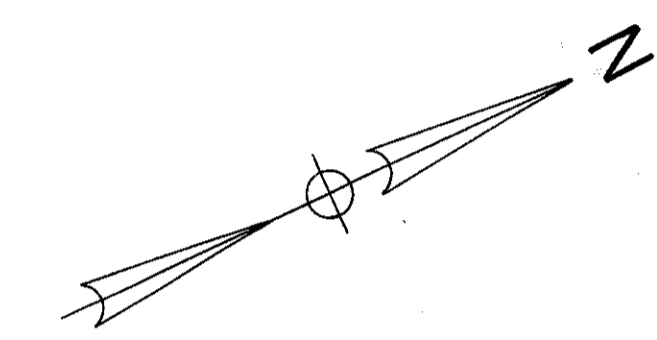
PLAN FOR DRAIN INLET AT S. ABUT. OF NORTH BOUND ROADWAY



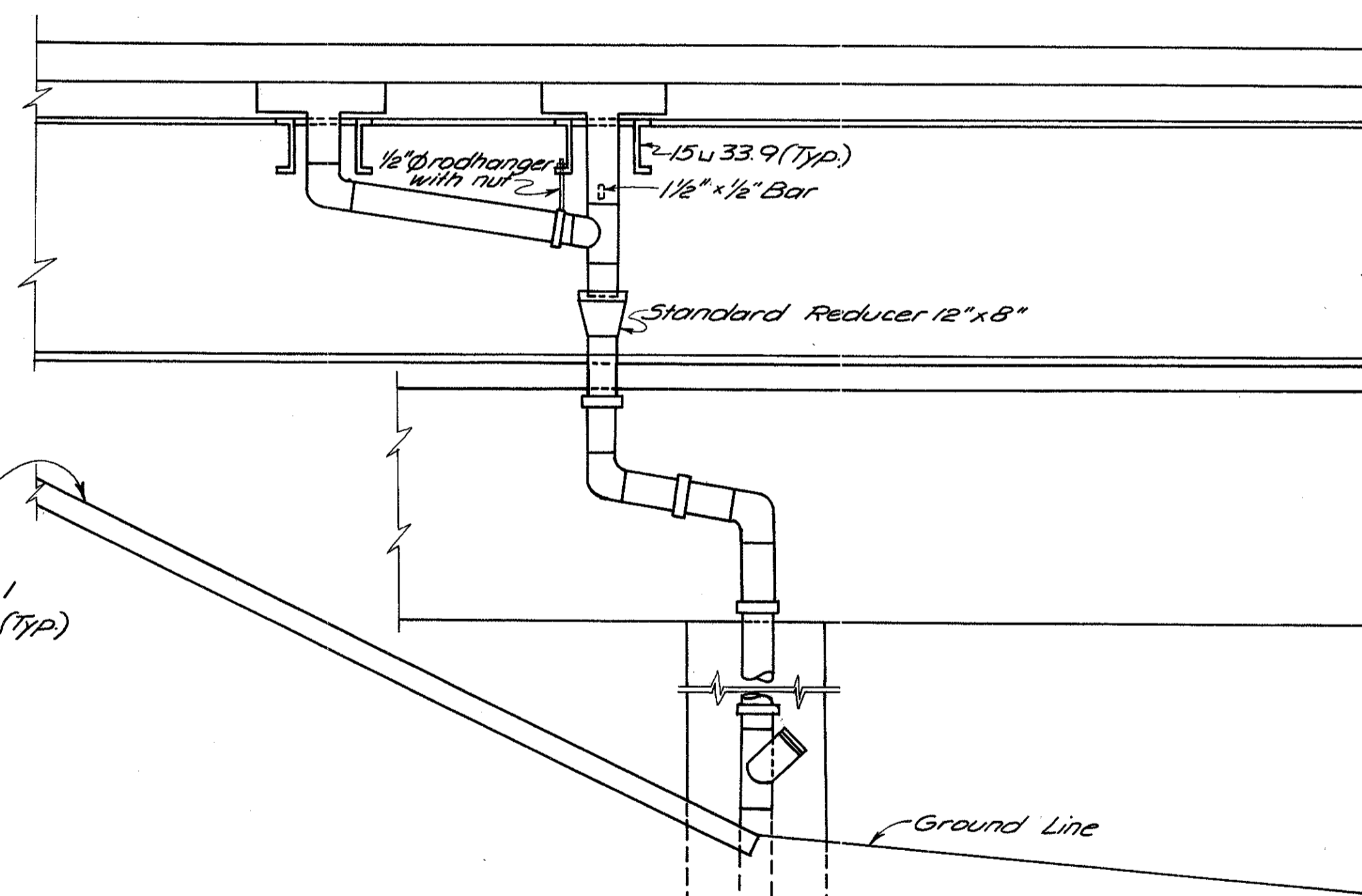
PLAN FOR DRAIN INLET AT PIER NO. 1 OF NORTH BOUND ROADWAY



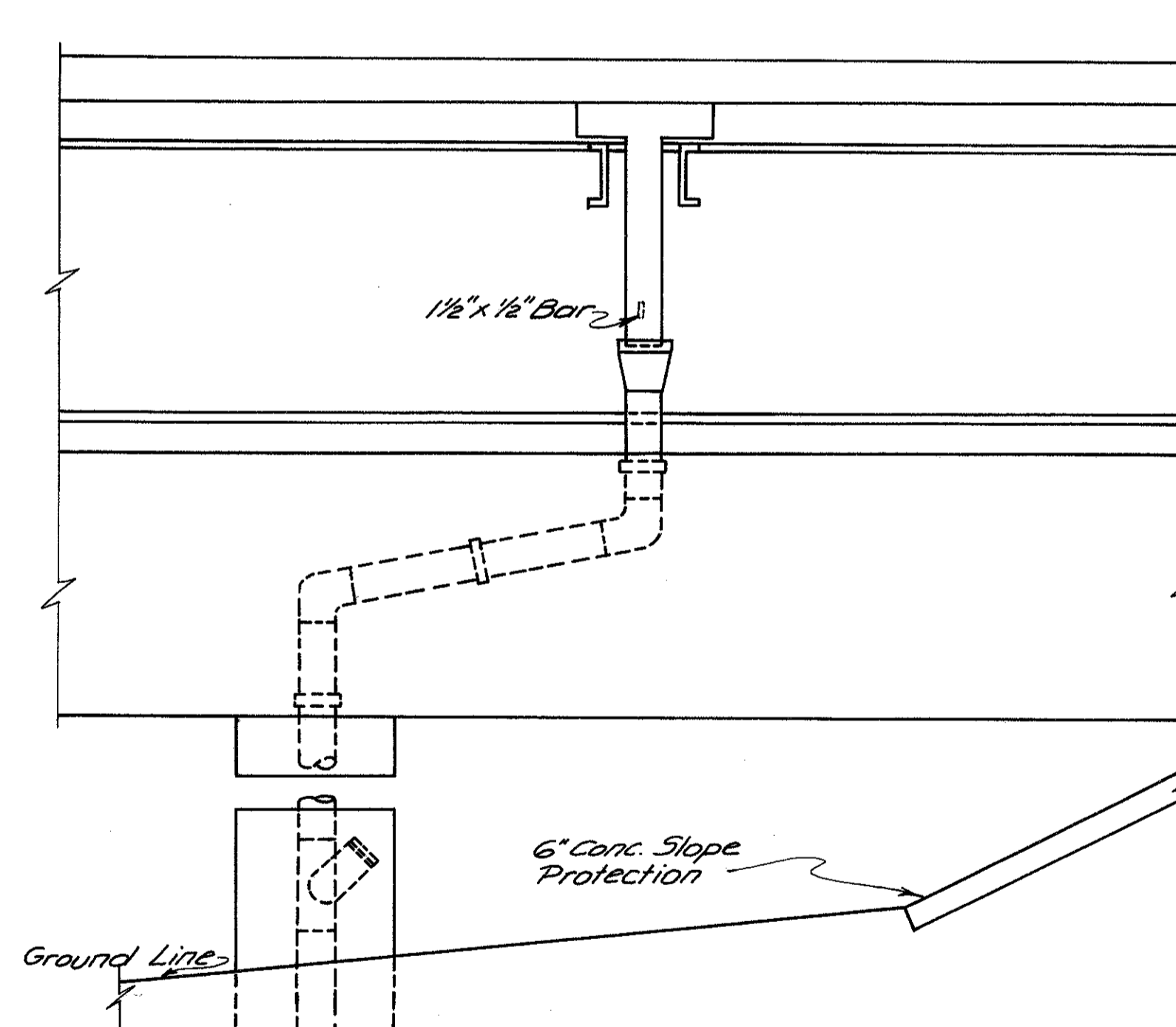
PLAN FOR DRAIN INLET AT PIER NO. 2 OF NORTH BOUND ROADWAY



SECTION D-D



SECTION E-E



SECTION F-F

For elevation see Roadway Drainage Sheet No. 95 thru 97. (Typ)

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FEB 20 1995

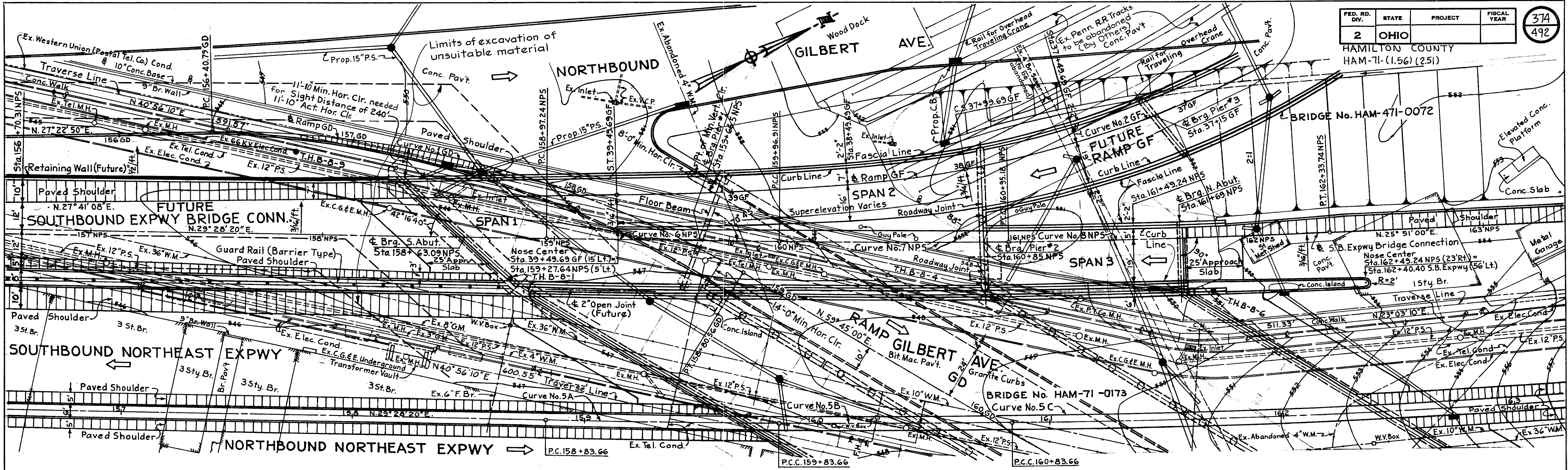
Work this sheet with sheet No. 368

HAZELT & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

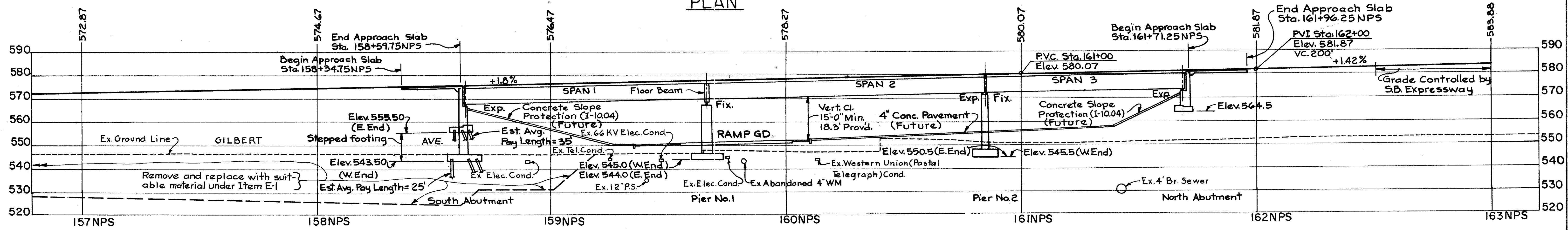
DRAINAGE DETAILS
BRIDGE No. HAM-71-0173

H & E BRIDGE No. 8

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
M. J. E.	B. Sch.	J. H. O.		5-20-65	



PLAN



PROFILE ON BASE LINE NPS

NOTES
 S. Abutment shall be constructed under this contract. The remaining portions of the bridge will be constructed under a future contract.
 • Symbol denotes drill hole. For Test Boring Data, see sheets 1, 2 & 10 of 13. For Bench Marks, see sheet 40.

CURVE DATA		
CURVE No. 6 NPS PI Sta. 159+47.08 NPS Δ=0°15'01" D=0°15'04" R=22,816.90' L=99.67' T=49.83'	CURVE No. 7 NPS PI Sta. 160+46.05 NPS Δ=1°14'59" D=1°16'18" R=4505.57' L=98.27' T=49.14'	CURVE No. 8 NPS PI Sta. 161+64.47 NPS Δ=2°07'20" D=1°31'54" R=3740.75' L=138.56' T=69.29'
CURVE No. 2 GF PI Sta. 35+83.76 GF Δ=73°20'14" D=63°35'14" R=9'45'00" D=13'00'00" R=440.74' L=489.13' T=273.20' L=150.00'		

PROPOSED STRUCTURE
 TYPE: Continuous Plate Girder (Span 1 & 2) and Simple Span Steel Girder (Span 3) with Reinforced Concrete Deck and Substructure.
 SPANS: 117'-0", 118'-6", 84'-0", % Bearings measured along base line (NPS).
 ROADWAY: Varies (See Plan) with 1'-0" Curb west side and 3'-8" Safety Curb East Side.
 SKEW: Varies, (See Plan)
 LOADING FREQUENCY: CF-2000 (57) Adequate AASHTO alternate loading.
 WEARING SURFACE: 1" Monolithic Concrete
 APPROACH SLAB: AS-1-54 (25' long)
 ALIGNMENT: Varies, See Plan
 SUPERELEVATION: Varies, See Plan

1984 Traffic Count			
a) From IR-71	ADT 9300	DHV 1540	
b) From Ramp GF	7900	1010	
Total	17200	2550	

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SITE PLAN
 BRIDGE No. HAM-471-0072
 SOUTHBOUND EXPRESSWAY BRIDGE CONNECTION AND RAMP GF OVER RAMP GD

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
ALT.			H.A.Z.	JHO 5/20/65	

MICROFILMED
FEB 20 1965

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

375
492

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	SOUTH ABUTMENT
E-2	480	Sq. Ft.	Steel Sheet Piling Left in Place (Min. Sect. Modulus of 7in. ³ per foot of wall)	480
E-2	440	Cu. Yds.	Unclassified Excavation	440
S-1	144	Cu. Yds.	Class "E" Concrete, Abutments above Footings	144
S-1	96	Cu. Yds.	Class "E" Concrete, Footing	96
S-3	7	Sq. Yds.	Type "B" Waterproofing	7
S-3	20	Lin. Ft.	Waterproofing, Premolded Sealing Strip	20
S-4	20314	Lbs.	Reinforcing Steel	20314
S-9	20	Lin. Ft.	1" Preformed Expansion Joint Filler (M10.02, Type I)	20
S-18	1420	Lin. Ft.	Steel Piles, IOBP42	1420
S-29	58	Cu. Yds.	Porous Back-fill	58
S-29	18	Lin. Ft.	8" Bituminous Coated Helical Perforated Corrugated Metal Pipe [Sec. M-64(h-c)] (Including Specials)	18
S-29	20	Lin. Ft.	8" Bituminous Coated Helical Corrugated Metal Pipe [Sec. M-64(h-c)] (Including Specials)	20
S-29	87	Lin. Ft.	8" Bituminous Coated Helical Perforated Corrugated Metal Pipe [Sec. M-64(h-c)] (Including Specials and Sand)	87
S-29	8	Lin. Ft.	12" Reinforced Concrete Sewer Pipe [Sec. M-66(a)]	8
I-10	425	Sq. Yds.	Concrete Slope Protection	425
E-2	Lump	Sum	Cofferdams, Cribbs and sheeting	Lump

GENERAL NOTES

PILES shall be driven with a hammer of not less than 11,000 ft. lbs per blow to firm contact with limestone. If the length of penetration is approximately equal to the depth of limestone according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Sec. S-18.05 is not less than the following value for a pile hammer of the indicated energy rating.

50 Tons per pile for South Abutment using an 11,000 ft. lb. hammer.
45 Tons per pile for South Abutment using a 15,000 ft. lb. hammer.

If the energy rating of the hammer is between the rating as shown above, the required formula capacity shall be determined by interpolation. The design load is 35 Tons per pile.

ADDITIONAL NOTES: For additional notes see Notes 2, 6, & 7 Sh. No. 443.

Design Loading - CF = 2000(57)
Concrete Class E - basic unit stress, 1,133 psi.
Reinforcing Steel - ASTM A15, A16, A160, Deformed, Intermediate, or Hard Grade. Basic unit stress 20,000 psi.

FIRST TEST PILE: Item S-16 in the Estimated Quantities for Bridge No. HAM-71-0173 may be driven at either HAM-71-0173 or HAM-471-0072. Payment will be made for only one first test pile.

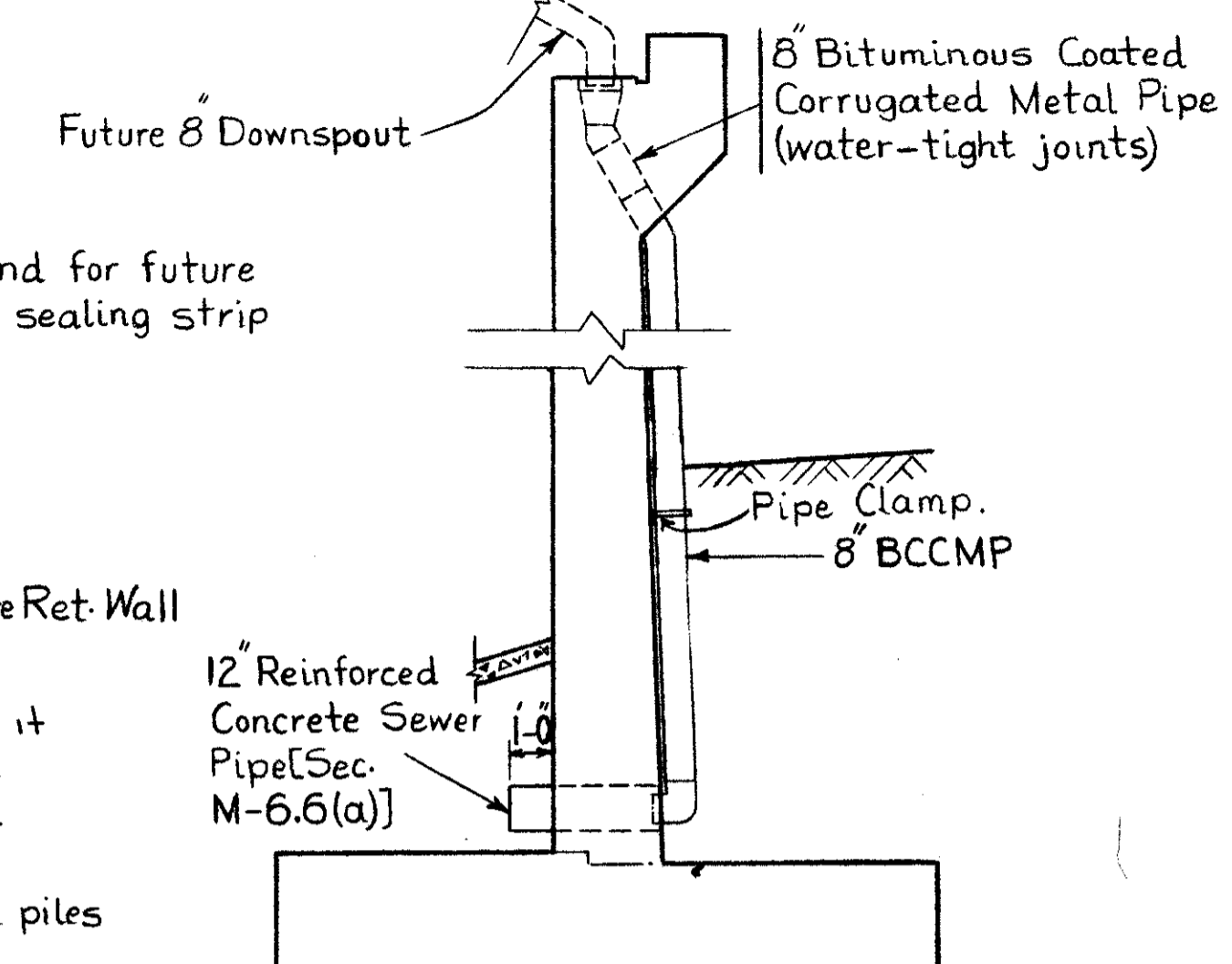
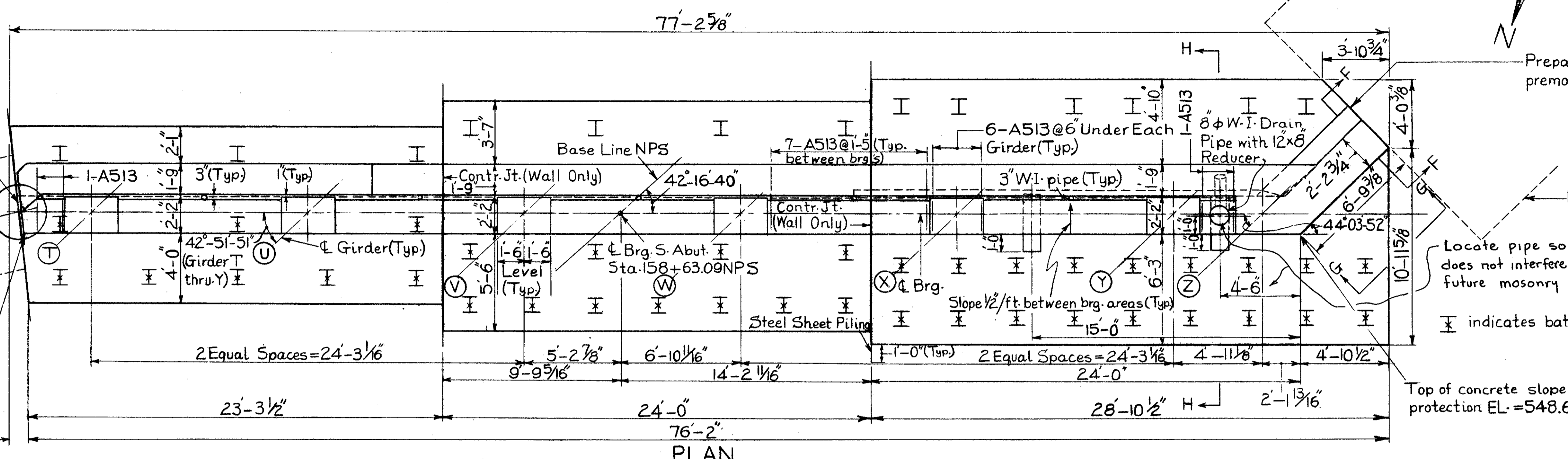
MICROFILMED
FEB 20 1985

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
ESTIMATED QUANTITIES AND GENERAL NOTES BRIDGE No. HAM-471-0072				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	⊕		W.L. 3-5-65	JHO 5/20/65

HAMILTON COUNTY
HAM-71-(1.56)-(2.51)

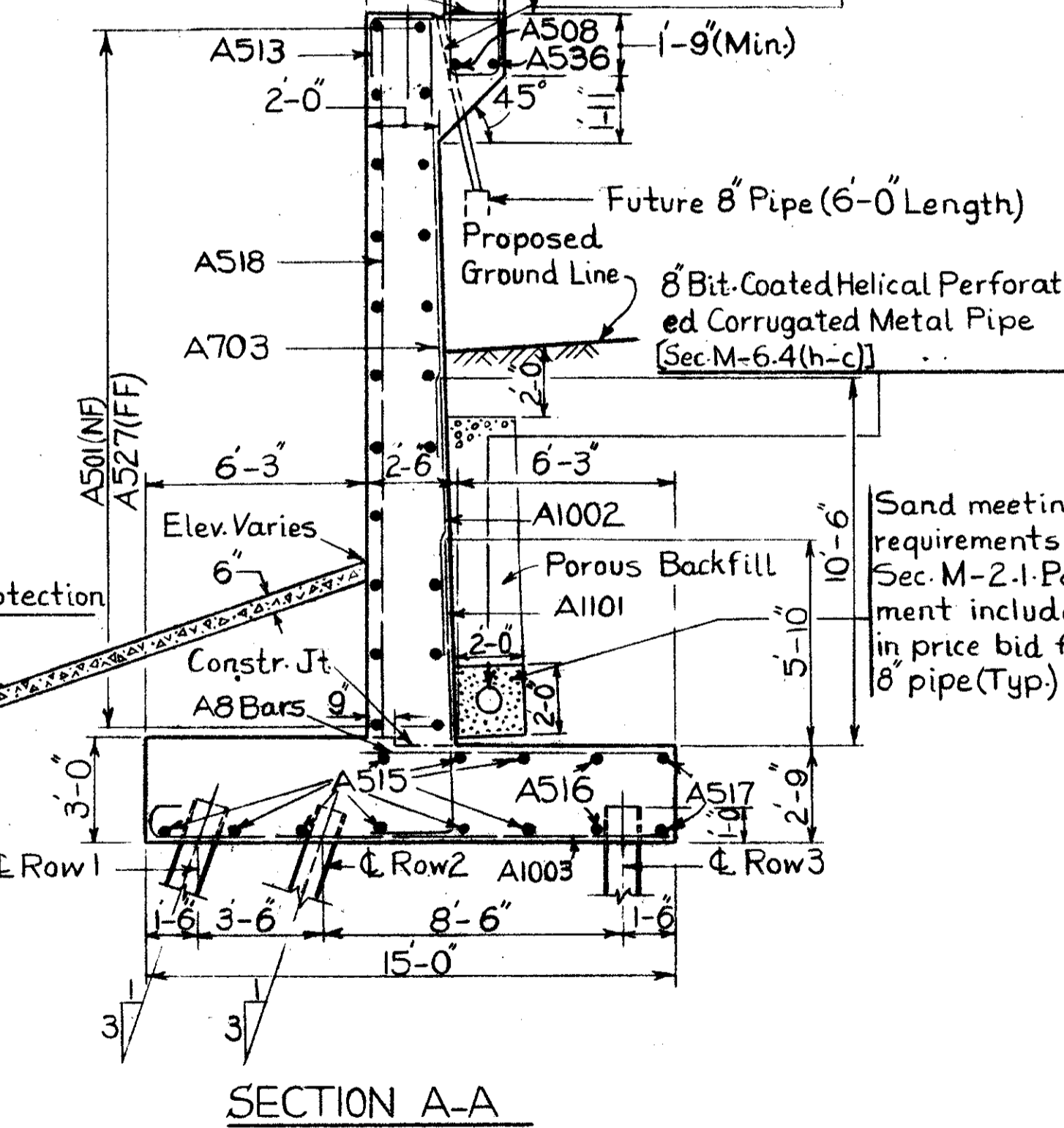
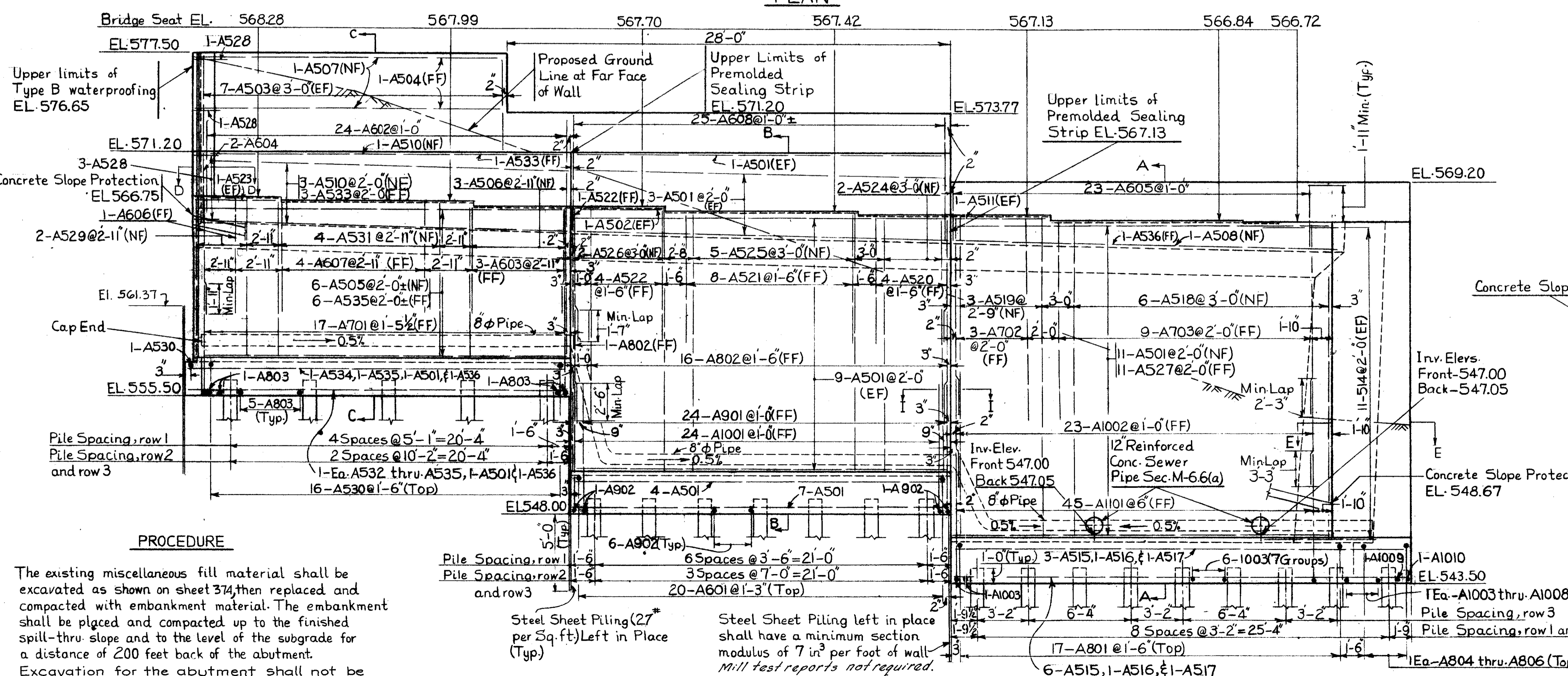
Expansion Jt. with Jt. Filler and Type B Waterproofing included with this structure for payment

Detail A
Bridge No. HAM-71-0173 (S-ABUT.)
Top of concrete slope protection EL. 566.75



Concrete above Constr. Jt. to be removed for future construction

3" W.I. pipe 5'-0" long located midway between brg. areas as shown on plan include with abutment concrete for payment (Typ)



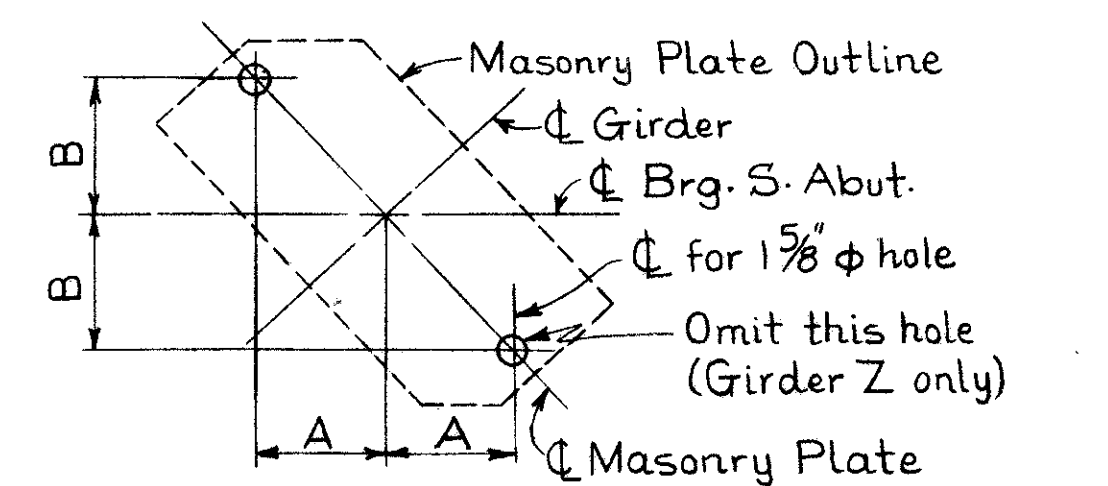
Work this sheet with sheet 377

For Notes see Sheet 377

MICROFILMED FEB 20 1965

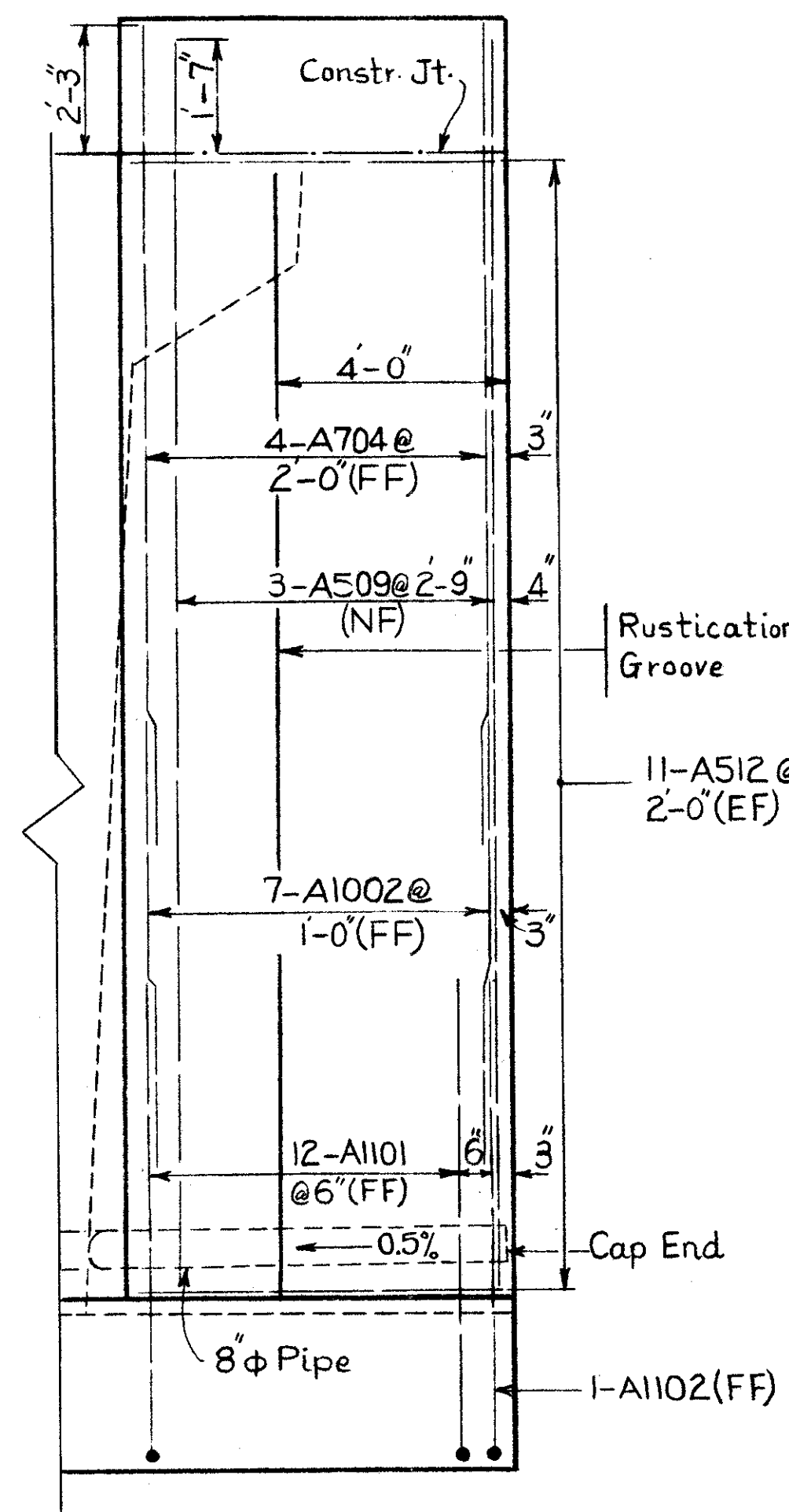
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	FN		W.L.	JHO	
			3-5-65	5/10/65	

SOUTH ABUTMENT
BRIDGE NO. HAM-471-0072

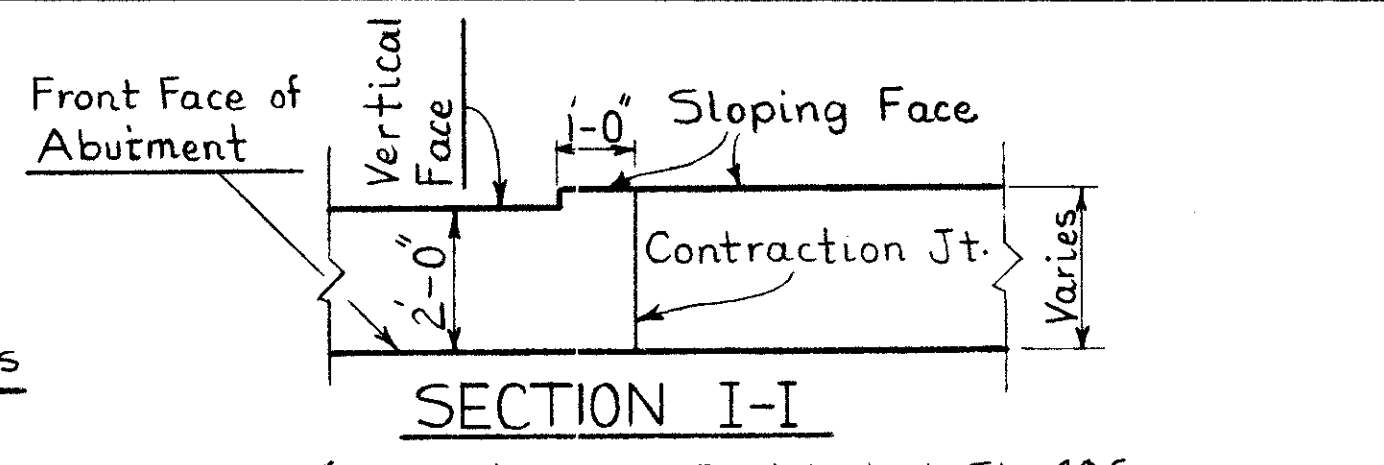


PROPOSED ANCHOR ROD DET.

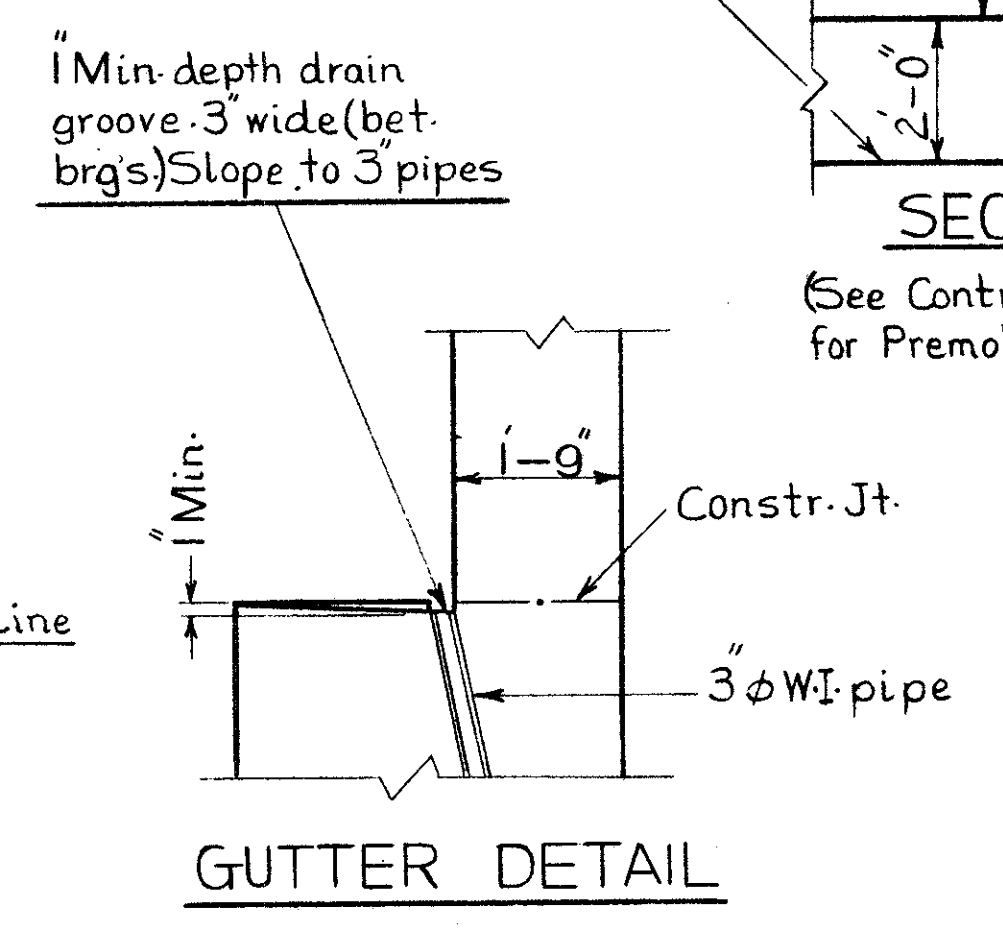
GIRDER	A	B
T thru Y	7 7/16	8 7/16
Z	8"	8 1/4"



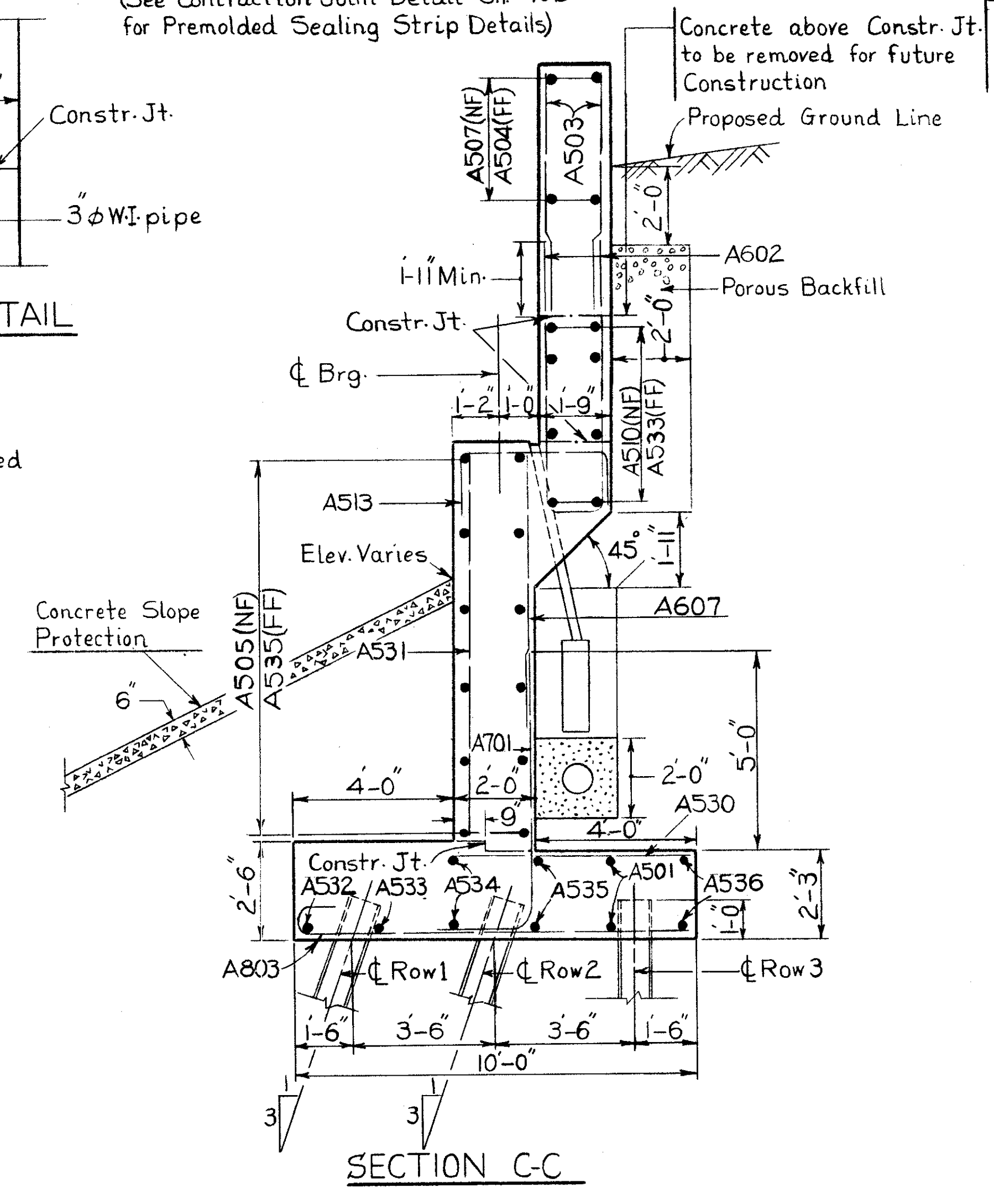
ELEV. G-G



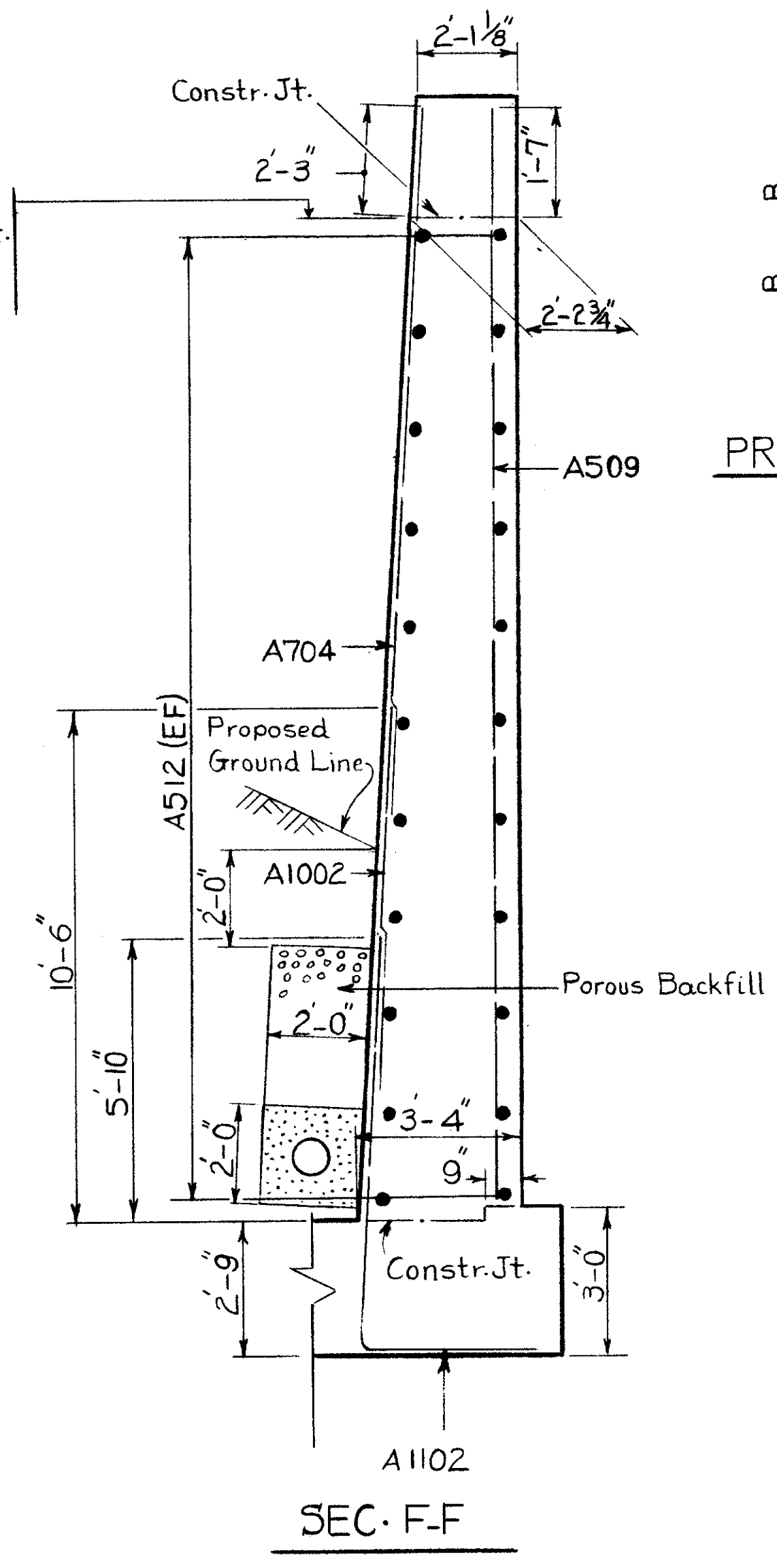
SECTION I-I
(See Contraction Joint Detail Sh. 406 for Premolded Sealing Strip Details)



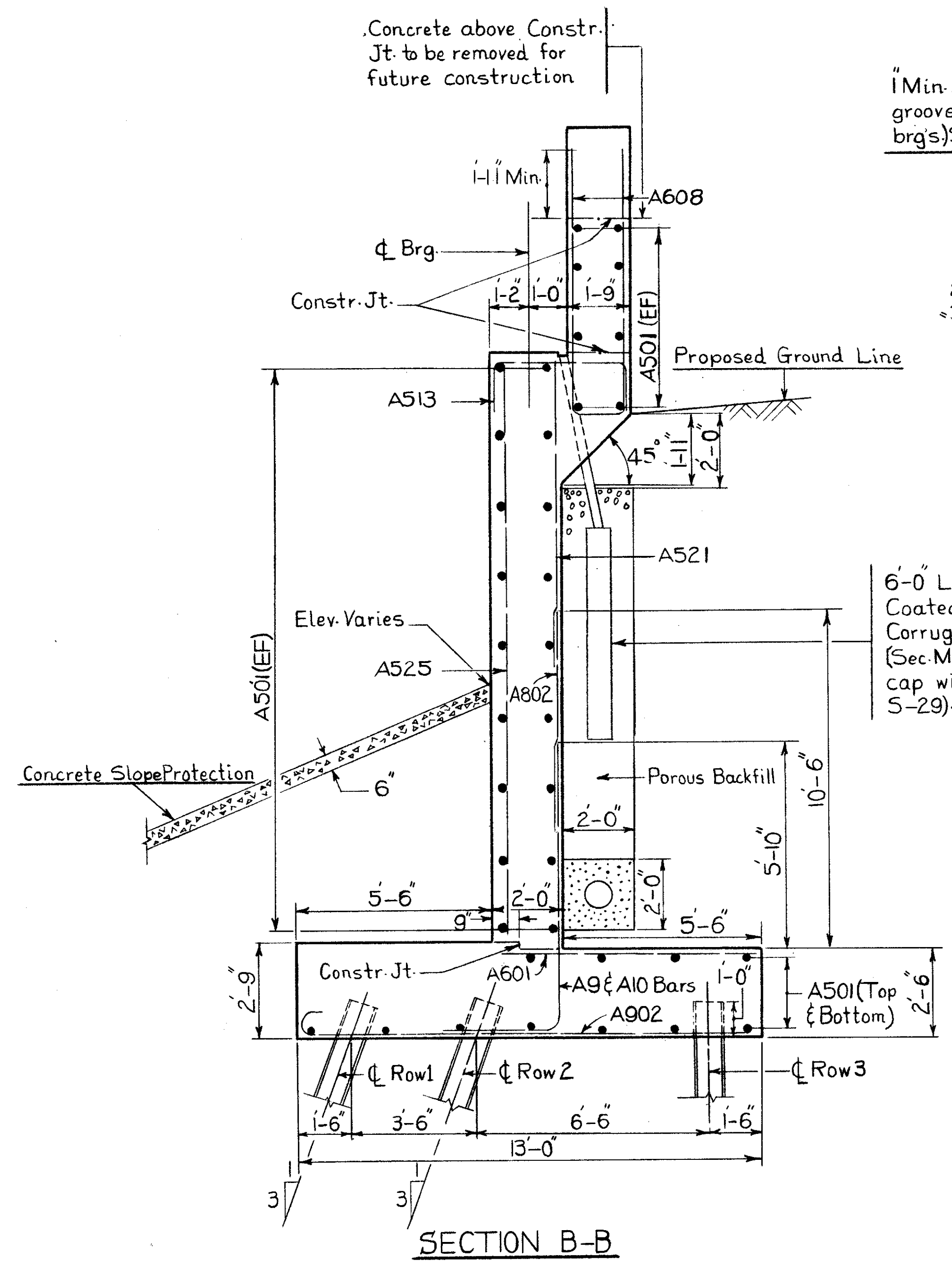
GUTTER DETAIL



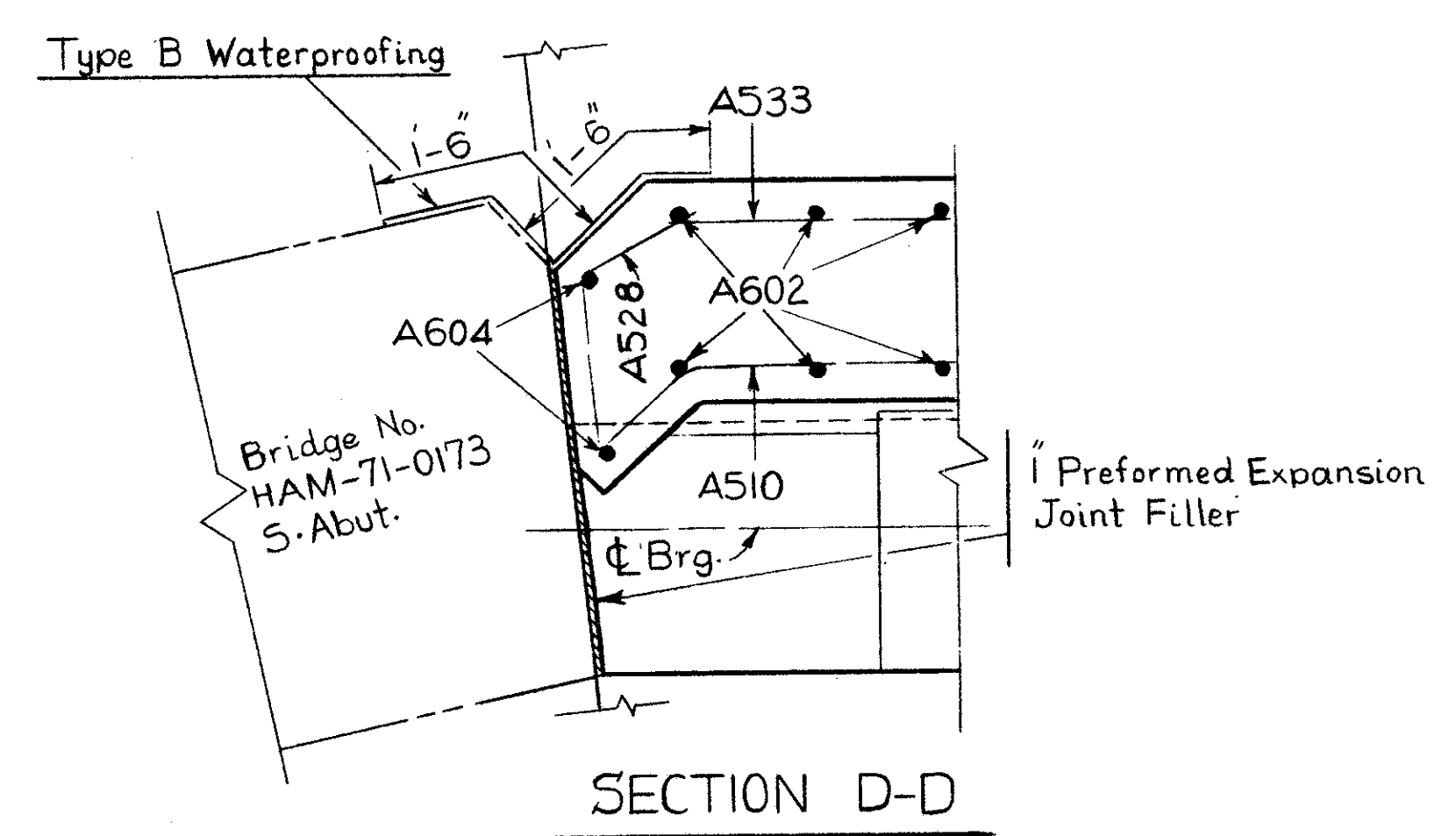
SECTION C-C



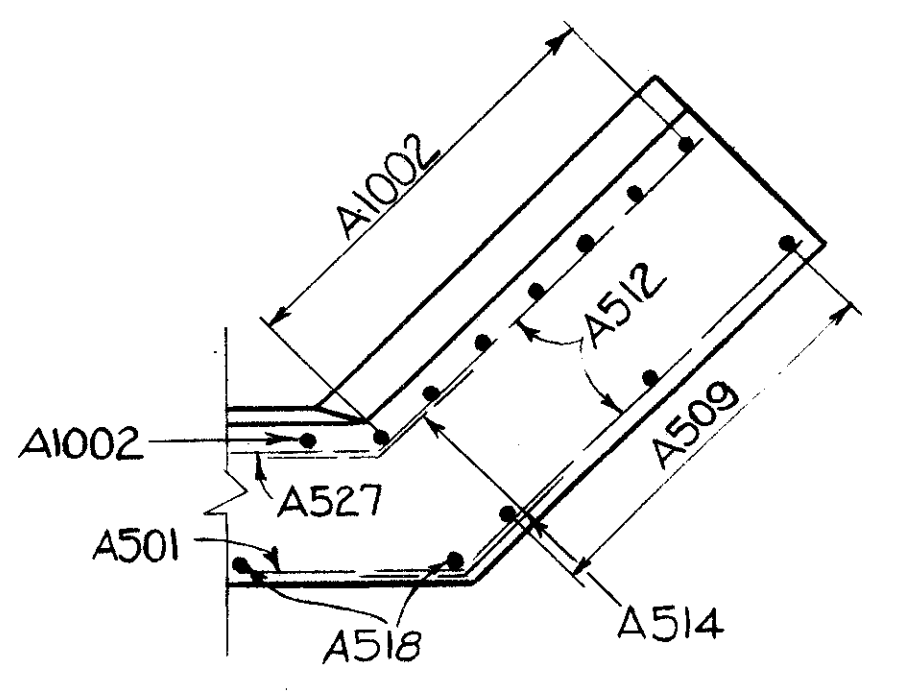
SEC. F-F



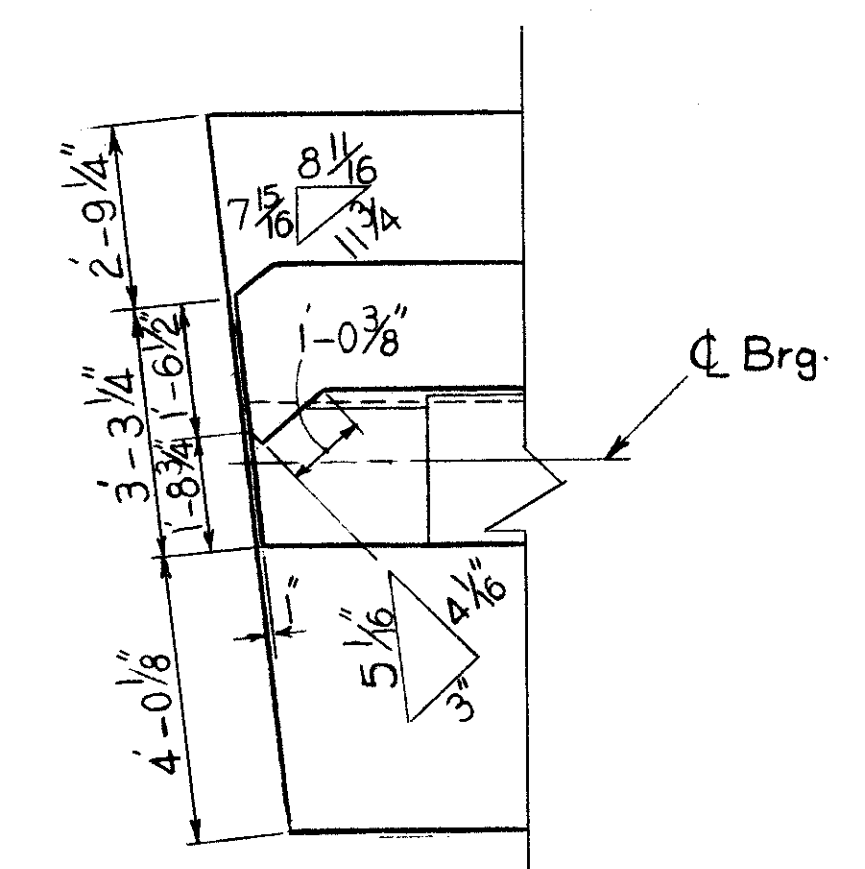
SECTION B-B



SECTION D-D



SEC. EE



DETAIL A

NOTES

- EF denotes Each Face
- NF denotes Near Face
- FF denotes Far Face
- Provide 3" clearance to reinforcing steel in footing and 2" clearance to reinforcing steel in wall, minimum.
- Special care shall be taken in placing steel in the bridge seat so that it will not interfere with drilling of future anchor rod holes.
- For Contraction Joint Detail see sheet 406
- For Rustication Groove Detail see sheet 373
- All piles are 10 BP 42
- All concrete shall be Class "E" Concrete
- For Abutment Drainage Clamp Detail see sheet 393

Work this sheet with sheet 376

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SOUTH ABUTMENT
BRIDGE NO. HAM-471-0072

MICROFILMED
FEB 20 1985

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	FN		W.L. 3-5-65	JH 5/20/65	

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

SOUTH ABUTMENT								
MARK	TYPE	a	b	c	d	NO.	LENGTH	WEIGHT
A501	Str.					50	23'-8"	1234
A502	Str.					2	5'-8"	12
A503	Str.					14	6'-8"	90
A504	Str.					2	19'-1"	40
A505	Str.					6	23'-4"	146
A506	Str.					3	9'-6"	30
A507	20	0'-7 1/2"	18'-10"	0'-11"	0'-8"	2	19'-9"	41
A508	Str.					1	22'-2"	23
A509	Str.					3	21'-9"	68
A510	20	0'-7 1/2"	22'-10"	0'-11"	0'-8"	4	23'-9"	99
A511	Str.					2	5'-11"	12
A512	Str.					22	6'-3"	144
A513	1	3'-7"	1'-7"			81	6'-6"	549
A514	20	1'-2 1/2"	1'-9"	1'-9"	1'-3"	22	3'-6"	81
A515	Str.					9	28'-5"	267
A516	Str.					2	26'-10"	56
A517	Str.					2	24'-10"	52
A518	Str.					6	20'-1"	126
A519	Str.					3	20'-6"	64
A520	Str.					4	7'-7"	32
A521	Str.					8	7'-10"	65
A522	Str.					5	8'-2"	43
A523	Str.					2	5'-1"	11
A524	Str.					2	16'-2"	34
A525	Str.					5	16'-6"	86
A526	Str.					2	16'-9"	35
A527	Str.					11	22'-9"	261
A528	20	0'-7"	1'-5"	0'-9"	0'-5 1/4"	5	2'-2"	11
A529	Str.					2	10'-1"	21
A530	Str.					17	5'-9"	102
A531	Str.					4	9'-10"	41
A532	Str.					1	22'-11"	24
A533	Str.					5	23'-1"	120
A534	Str.					2	23'-3"	43
A535	Str.					8	23'-6"	196
A536	Str.					3	23'-10"	75
A601	Str.					20	7'-3"	218
A602	1	1'-5"	7'-2"			24	15'-5"	556
A603	Str.					3	6'-9"	30
A604	Str.					2	10'-10"	33
A605	1	1'-5"	3'-9"			23	8'-7"	297
A606	Str.					2	7'-4"	22
A607	Str.					4	7'-0"	42
A608	1	1'-5"	7'-9"			25	16'-7"	623
A701	18	2'-3"	7'-0"			17	9'-1"	316
A702	Str.					3	12'-6"	77
A703	Str.					9	12'-1"	222
A704	Str.					4	12'-4"	117

SOUTH ABUTMENT								
MARK	TYPE	a	b	c	d	NO.	LENGTH	WEIGHT
A801	Str.					17	8'-6"	386
A802	Str.					17	7'-2"	325
A803	44	9'-6"				24	10'-7"	678
A804	Str.					1	7'-7"	20
A805	Str.					1	6'-0"	16
A806	Str.					1	4'-7"	12
A901	18	2'-10"	8'-1"			24	10'-8"	870
A902	44	12'-6"				40	13'-9"	1870
A1001	18	3'-3"	8'-1"			24	11'-1"	1145
A1002	Str.					30	7'-11"	1022
A1003	44	14'-6"				45	15'-11"	3082
A1004	44	14'-5"				1	15'-10"	68
A1005	44	14'-0"				1	15'-5"	66
A1006	44	13'-7"				1	15'-0"	65
A1007	44	13'-2"				1	14'-7"	63
A1008	44	12'-9"				1	14'-2"	61
A1009	44	11'-8"				1	13'-1"	56
A1010	44	10'-8"				1	12'-1"	52
A1101	19	3'-6 3/8"	3'-7"	0'-2 1/8"	8'-4"	57	11'-7"	3508
A1102	19	2'-11 1/8"	3'-0"	0'-2 1/8"	8'-4"	1	11'-0"	58
TOTAL WEIGHT = 20314								LBS.

REPLACEMENT BARS			
MARK	TYPE	NO.	LENGTH
RE501	Str.	1	5'-7"
RE601	Str.	1	5'-11"
RE701	Str.	1	6'-3"
RE801	Str.	1	6'-6"
RE901	Str.	1	6'-10"
RE1001	Str.	1	7'-3"
RE1101	Str.	1	7'-7"

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FEB 20 1985

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
REINFORCING STEEL LIST BRIDGE NO. HAM-471-0072					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
TLZ.			3-4-65	JH0 5/10/65	

NOTE: Work this sheet with sheets No. 446

NOTE: For Bar Bending Schedule see Typical Drawing, Sht. No. 446.

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

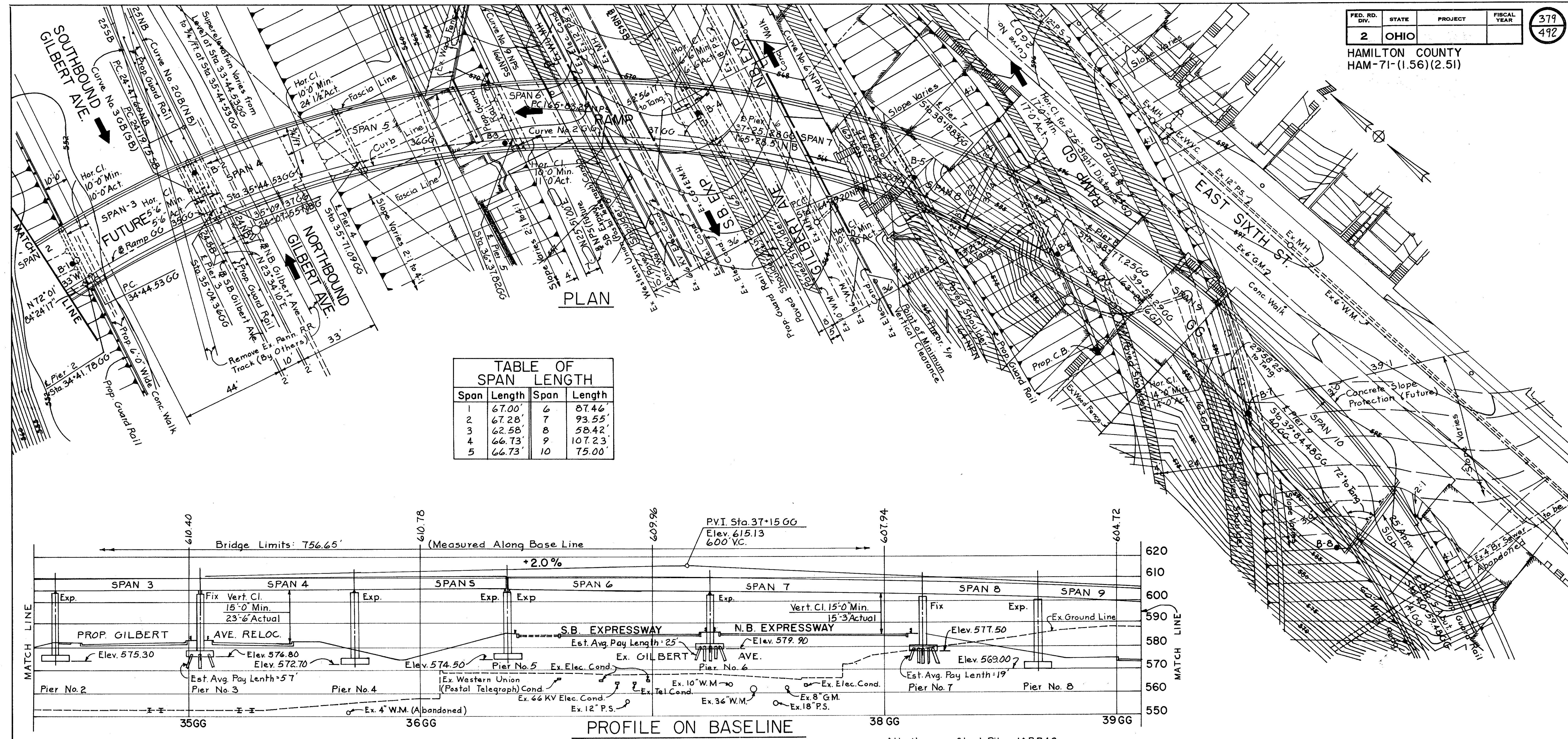
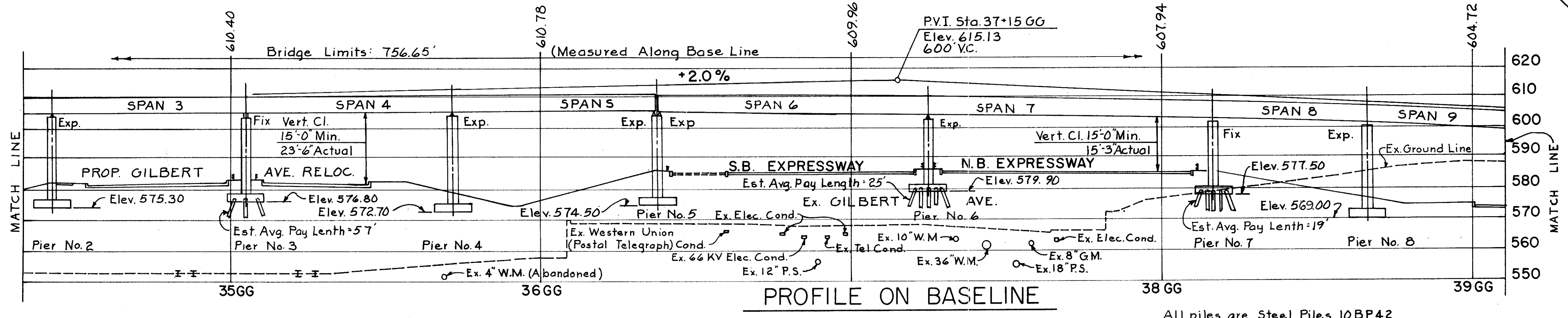


TABLE OF SPAN LENGTH

Span	Length	Span	Length
1	67.00'	6	87.46'
2	67.28'	7	93.55'
3	62.58'	8	58.42'
4	66.73'	9	107.23'
5	66.73'	10	75.00'



CURVE DATA
BASE LINE RAMP GG
 P.I. Sta. 40+40.54 GG
 P.C. Sta. 34+44.53 GG
 P.T. Sta. 42+28.30 GG
 $\Delta = 112^\circ 15' 56''$
 $D = 14^\circ 19' 26''$
 $R = 400.00'$
 $L = 783.77'$
 $T = 596.01'$

NOTES

- Symbol denotes drill hole
- For Test Boring Data, see sheet 5 & 10 of 13
- For Bench Marks, see sheet 40
- Piers 6, 7, 8, 9 and the footing only for Pier No. 3 shall be constructed under this contract. The remaining portions of the bridge will be constructed under a future contract.
- W. Abutment and Pier 1 are parallel
- Piers 2, 3, 4 and 5 are parallel

PROPOSED STRUCTURE

TYPE: Continuous Steel Plate Girders with reinforced concrete deck and substructure

SPANS: Lengths Vary, see tabulation

ROADWAY: 28'-0" f/f Parapets

CURBS: 1'-0"

LOAD FREQUENCY: CF=2000(57) Adequate for AASHO alternate loading

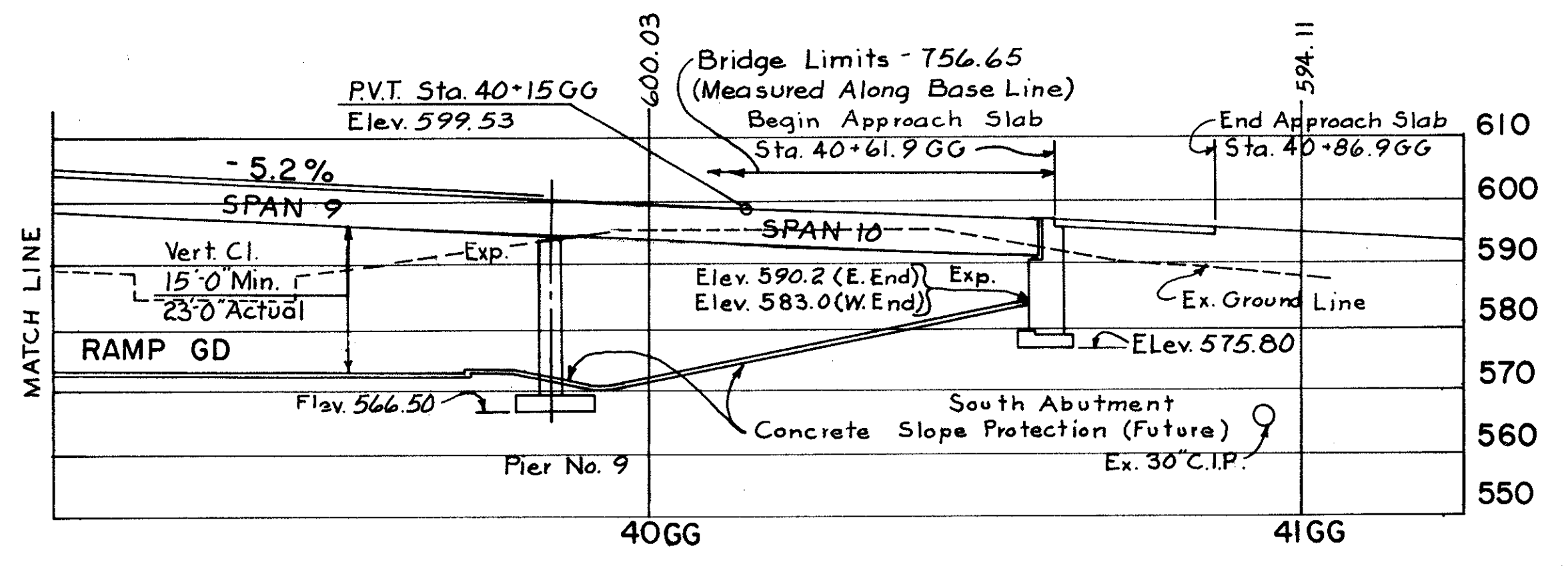
SKEW: Varies, See Plan

WEARING SURFACE: 1" Monolithic concrete

APPROACH SLABS: AS-1-54 25' long

ALIGNMENT: Varies, See Plan

SUPERELEVATION: Varies, See Plan



1984 Traffic Count	ATD=6800 DHV= 730
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO	
SITE PLAN	
BRIDGE No. HAM-71-0184	
FUTURE RAMP GG OVER GILBERT AVE. AND I-71	
H & E BRIDGE No. 10	
DESIGNED	DRAWN
AL.T.	AL.T.
TRACED	CHECKED
N.A.G.	J.H.O.
REVIEWED DATE	REVISED
5/10/65	

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

380
492

**HAMILTON COUNTY
HAM - 71 - (1.66) (2.51)**

GENERAL NOTES

DESIGN NOTES:

Design Loading CF=2000 (57)
Concrete Class C - Basic unit stress = 1333 psi
Concrete Class E - Basic unit stress = 1133 psi

Reinforcing Steel - ASTM A15, A16, A160 Deformed Intermediate or Hard Grade. Basic unit stress = 20,000 psi.
Except spiral reinforcement may be plain structural grade with basic unit stress = 18,000 psi.

STEEL PILES: shall be driven with a hammer of not less than 11,000 ft. lbs. per blow to firm contact with rock or shale. If the length of penetration is approximately equal to the depth of rock or shale according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Section S-18.05 is not less than the following value for a pile hammer of the indicated energy rating: 60 tons per pile using a 11,000 ft. lb hammer or 50 tons per pile using a 15,000 ft. lb hammer for Piers 3, 6 & 7.
If the energy rating of the hammer is between the rating shown above, the required formula capacity shall be determined by interpolation. The design of Piers 3, 6 & 7 is based on 35 Tons per pile.

ADDITIONAL NOTES: For additional notes see Notes 2, 6 & 7. General Notes, Typical Drawing No 443

FOUNDATION BEARING PRESSURE Piers 8 & 9 footings are designed for a maximum bearing pressure of 3 tons per sq. ft.

ESTIMATED QUANTITIES

Item	Total	Unit	Description	Pier 3	Pier 6	Pier 7	Pier 8	Pier 9	General
E-2	295	Cu.Yds.	Unclassified Excavation			40	150	105	
S-1	215	Cu.Yds.	Class 'C' concrete, piers above footings		37.7	58.2	58.8	60.3	
S-1	266	Cu.Yds.	Class "E" concrete, footings	39	46.7	69.1	53.3	57.9	
S-4	113,544	Lbs.	Reinforcing steel	6,710	18,718	28,017	29,478	30,616	
S-16	Lump	Sum	First test pile						
S-18	3049	Lin.Ft.	Steel piles 10 BP 42	1539	750	760			
E-2	50	Cu.Yds.	Rock (or Shale) Excavation				20	30	

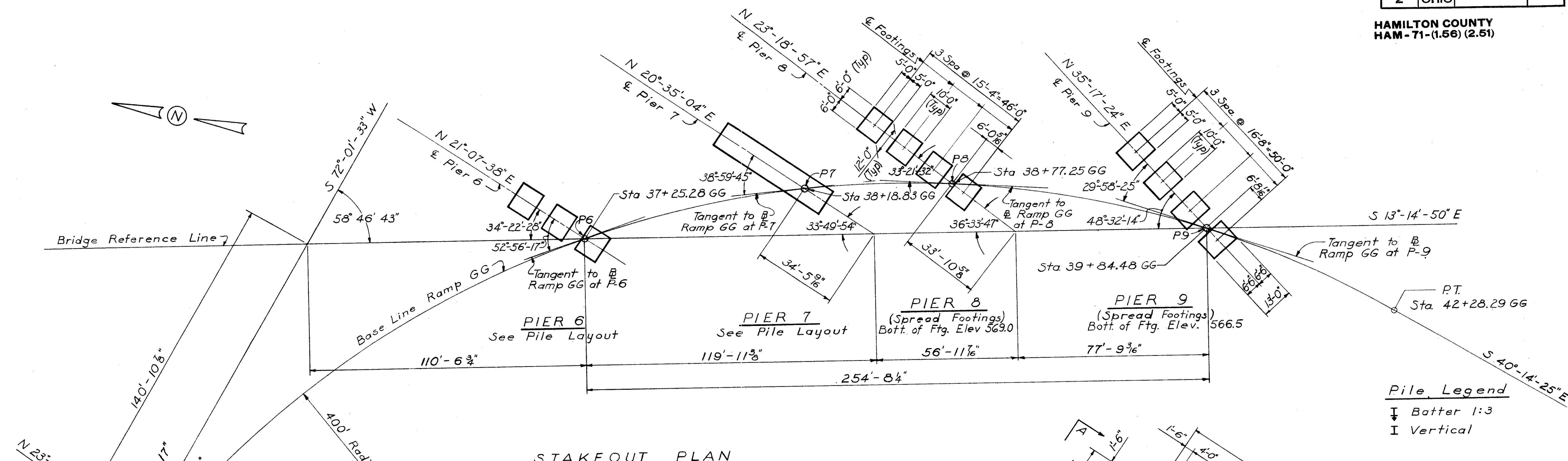
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FEB 20 1985

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
ESTIMATED QUANTITIES & NOTES					
BRIDGE NO. HAM - 71 - 0184					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISIONS
	S.U.		R.L.K. 4-15-65	JHO 5/20/65	

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

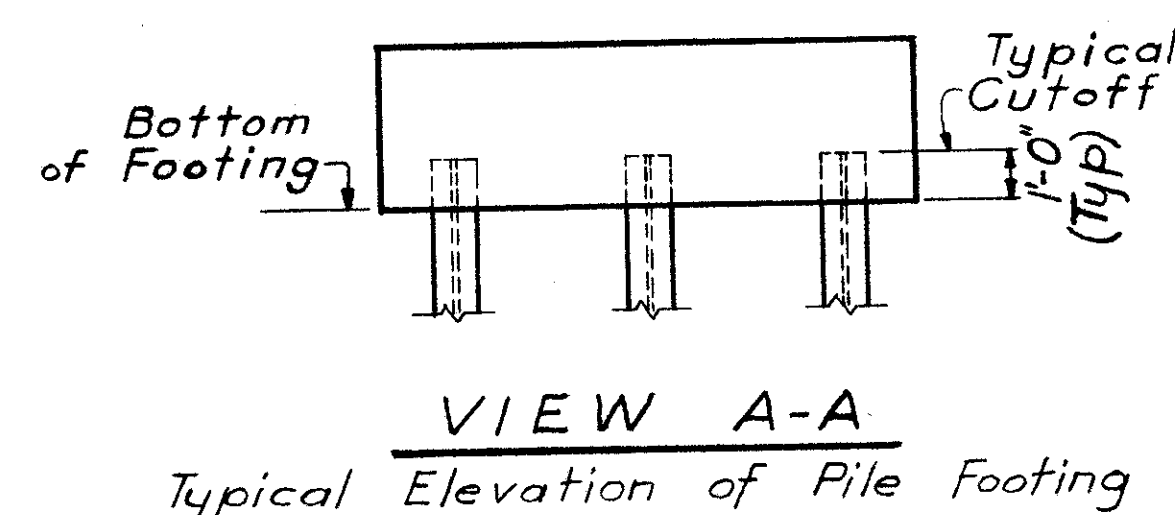
381
492

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

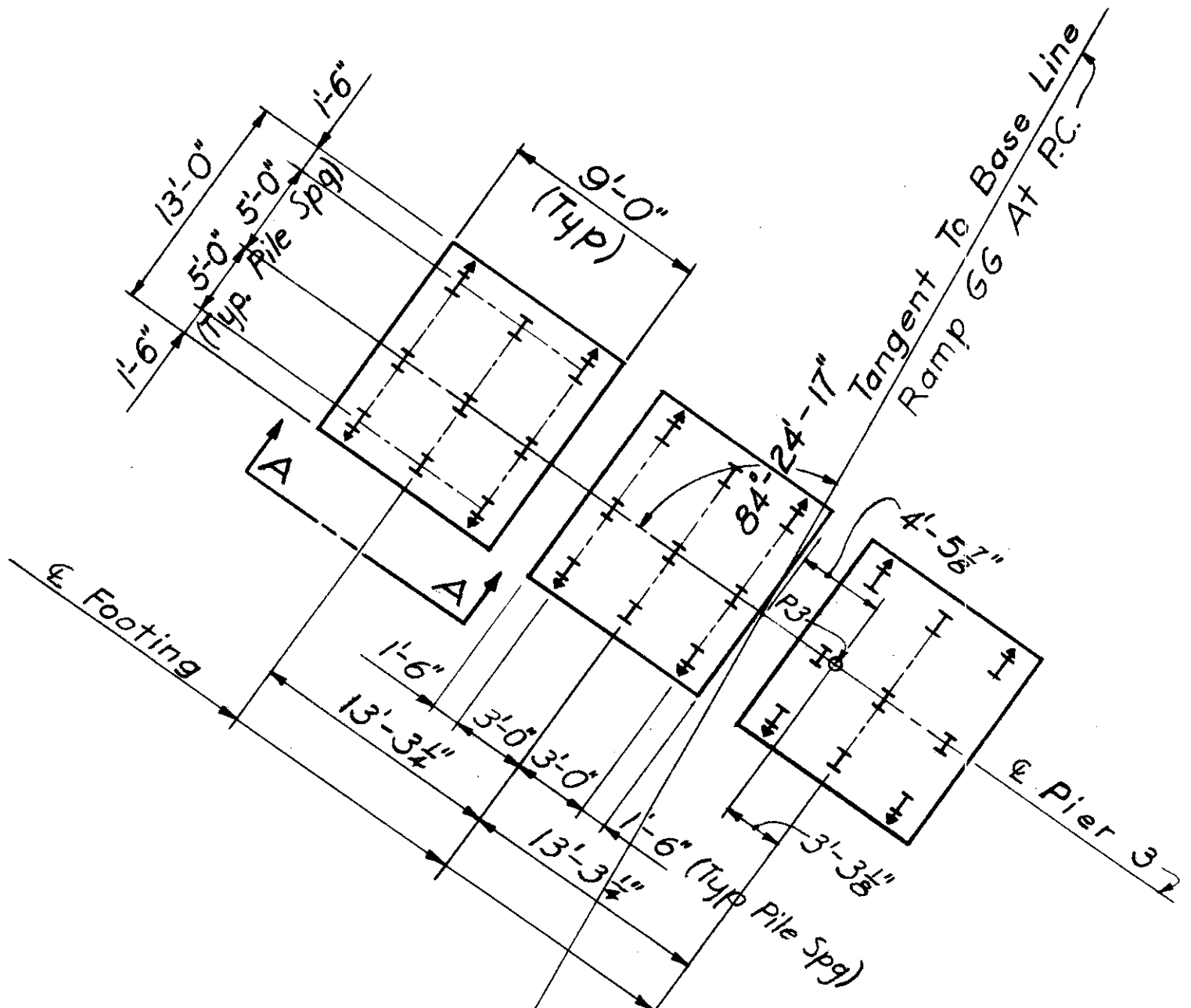


STAKEOUT PLAN

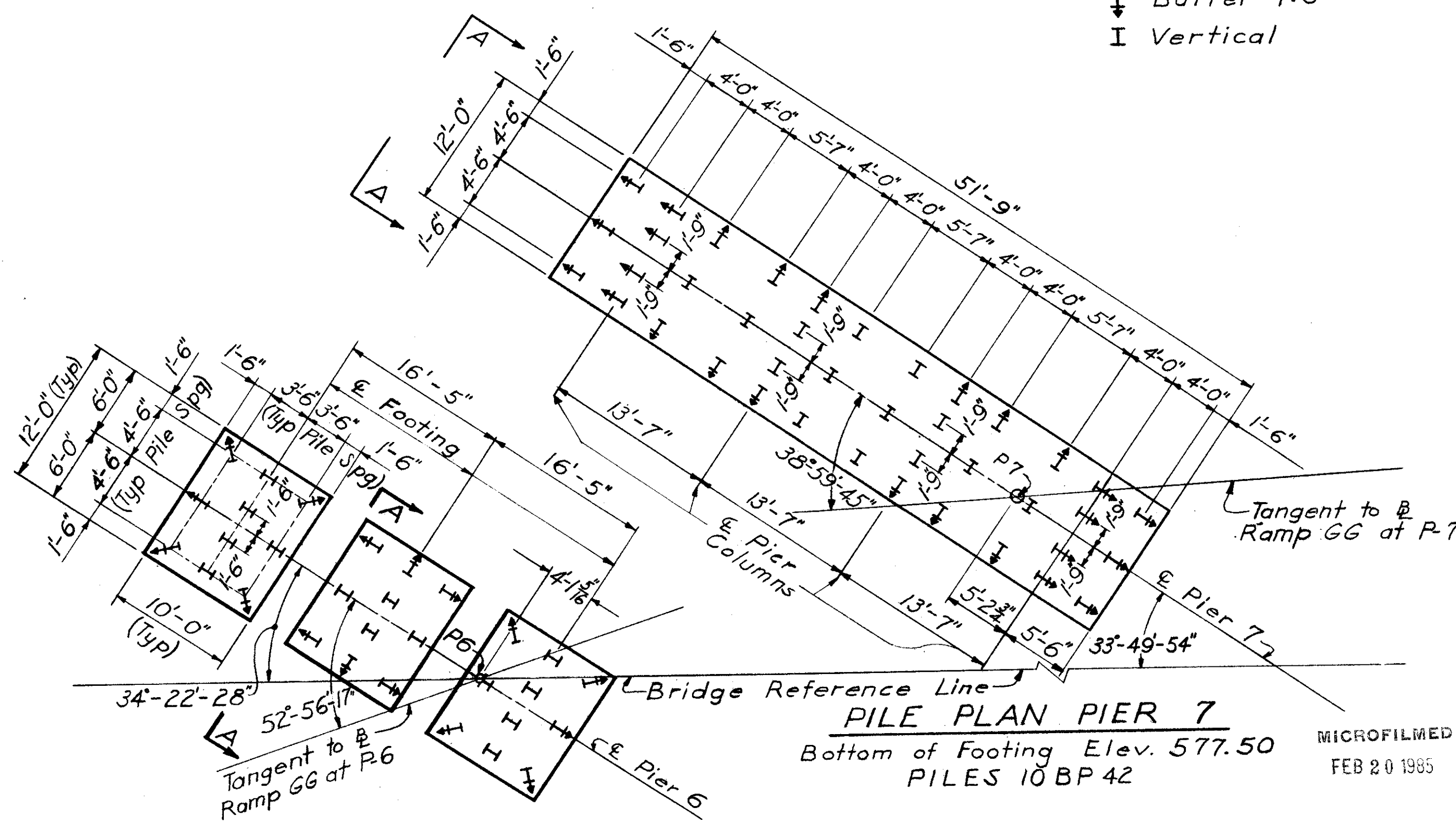
Pile Legend
 I Batter 1:3
 I Vertical



VIEW A-A
Typical Elevation of Pile Footing



PILE PLAN PIER 3
Bottom of Footing Elev. 576.80
PILES 10BP 42



PILE PLAN PIER 6
Bottom of Footing Elev. 579.90
PILES 10BP 42

PILE PLAN PIER 7
Bottom of Footing Elev. 577.50
PILES 10BP 42

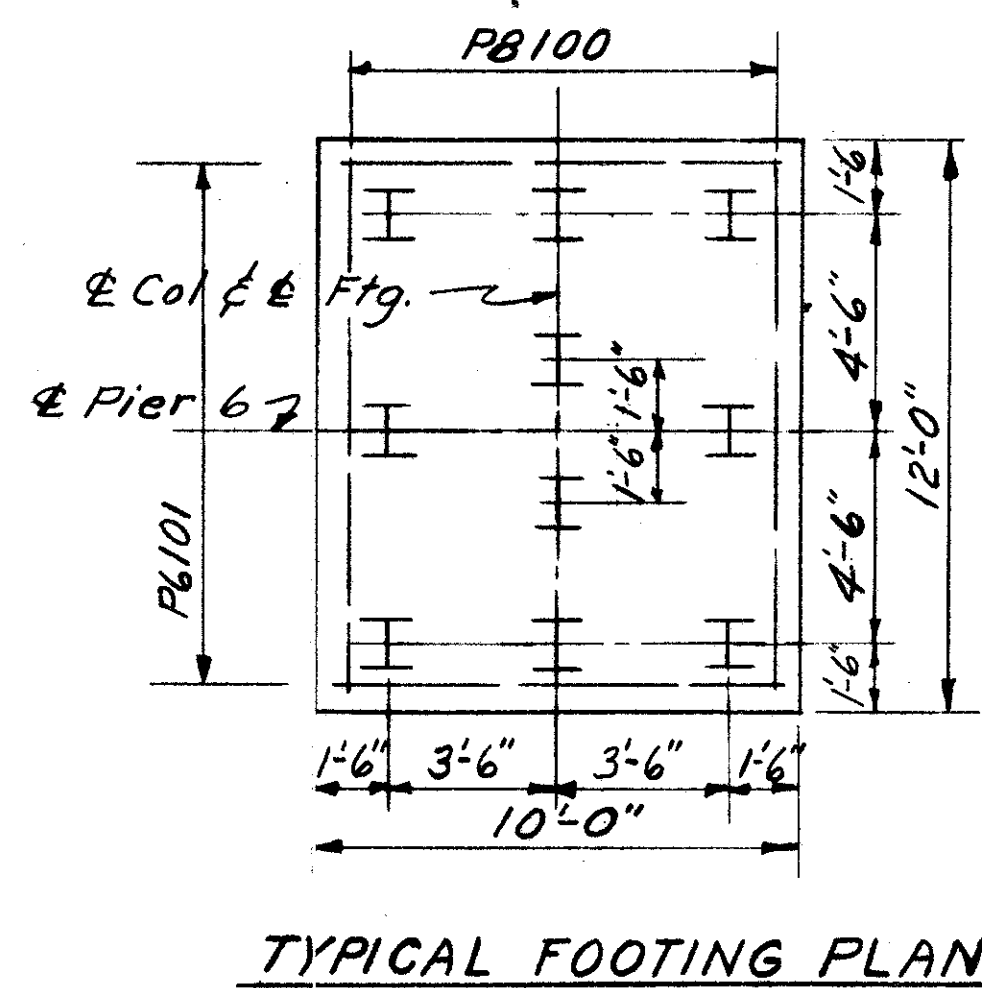
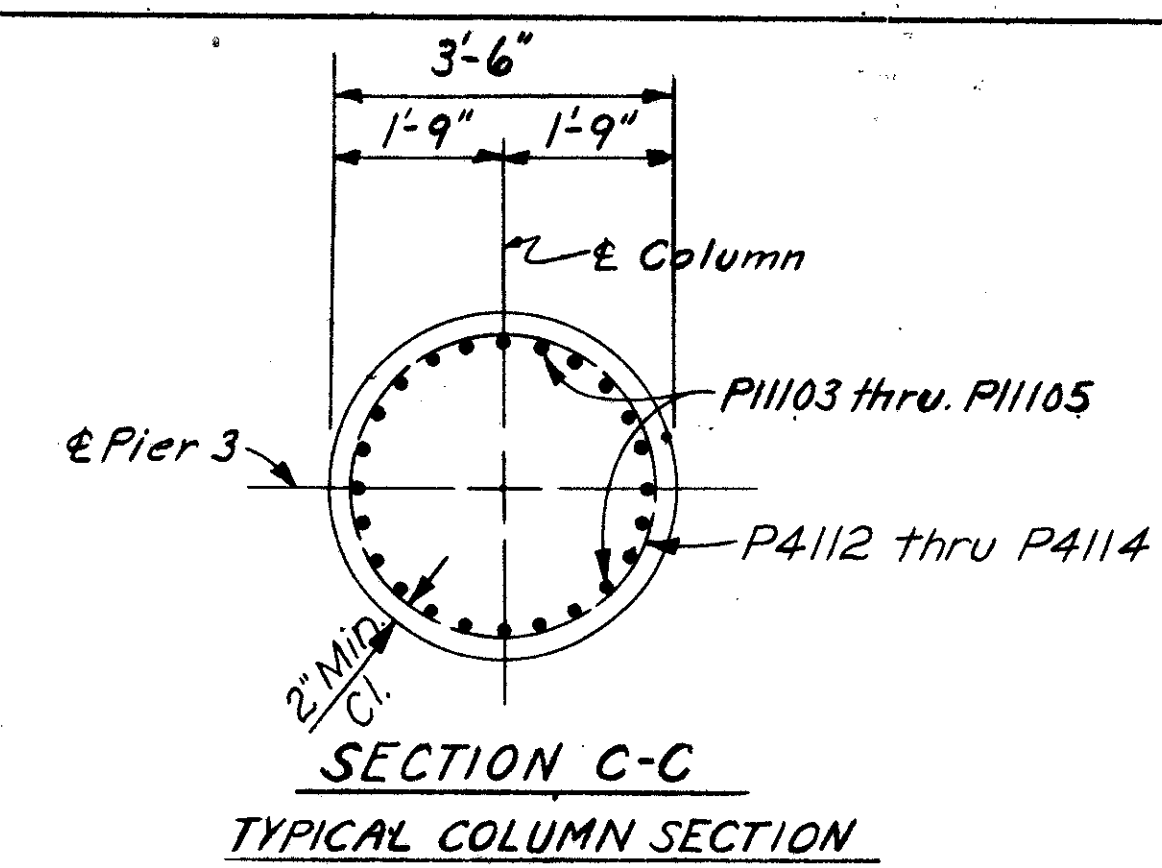
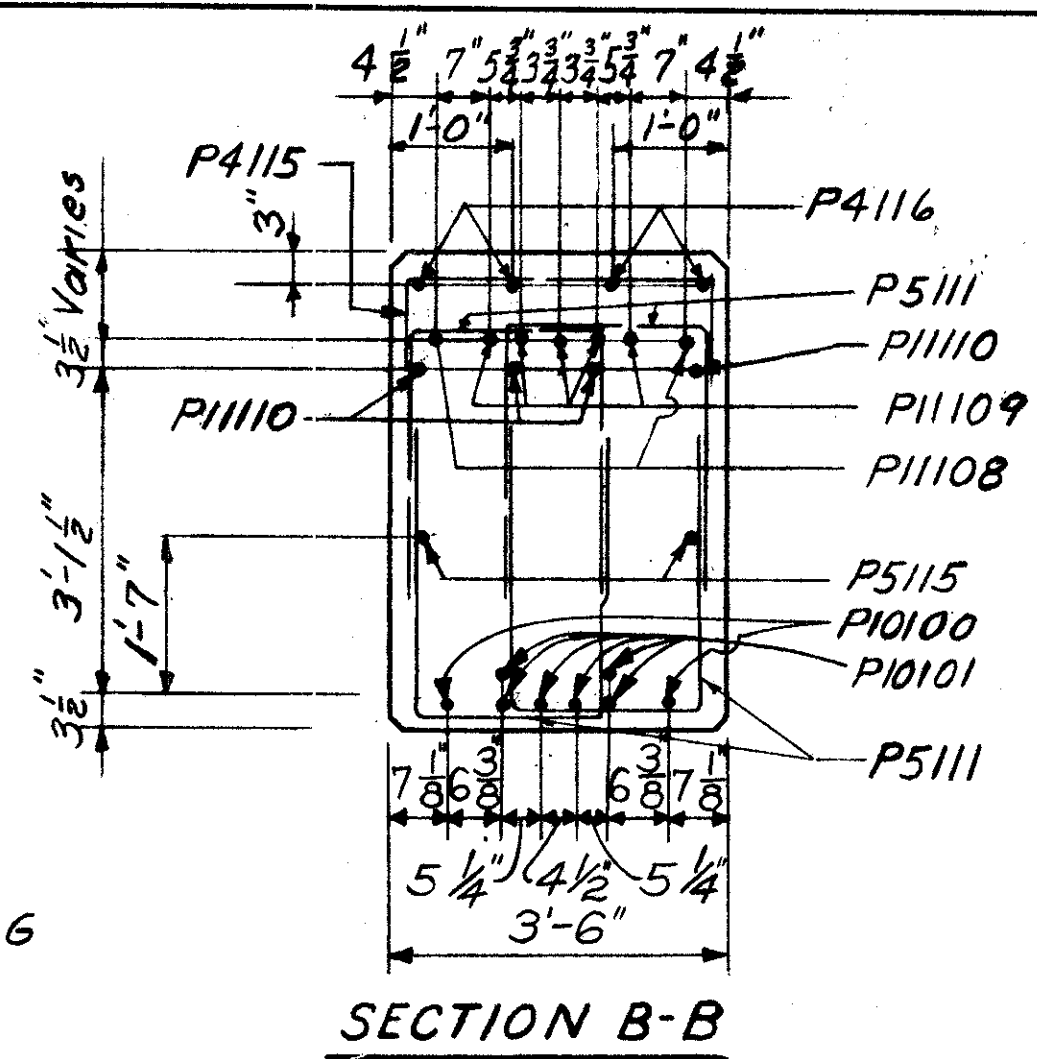
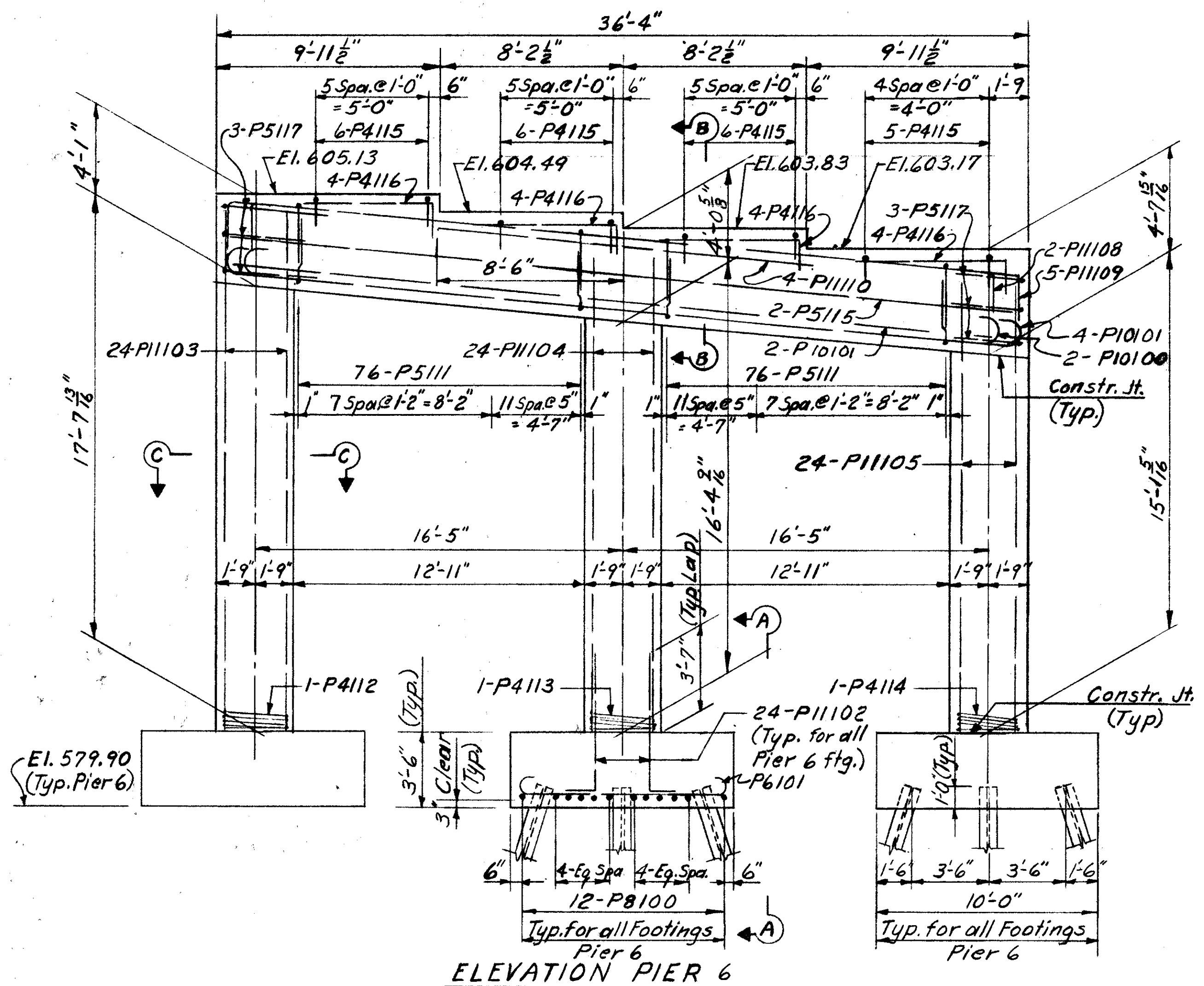
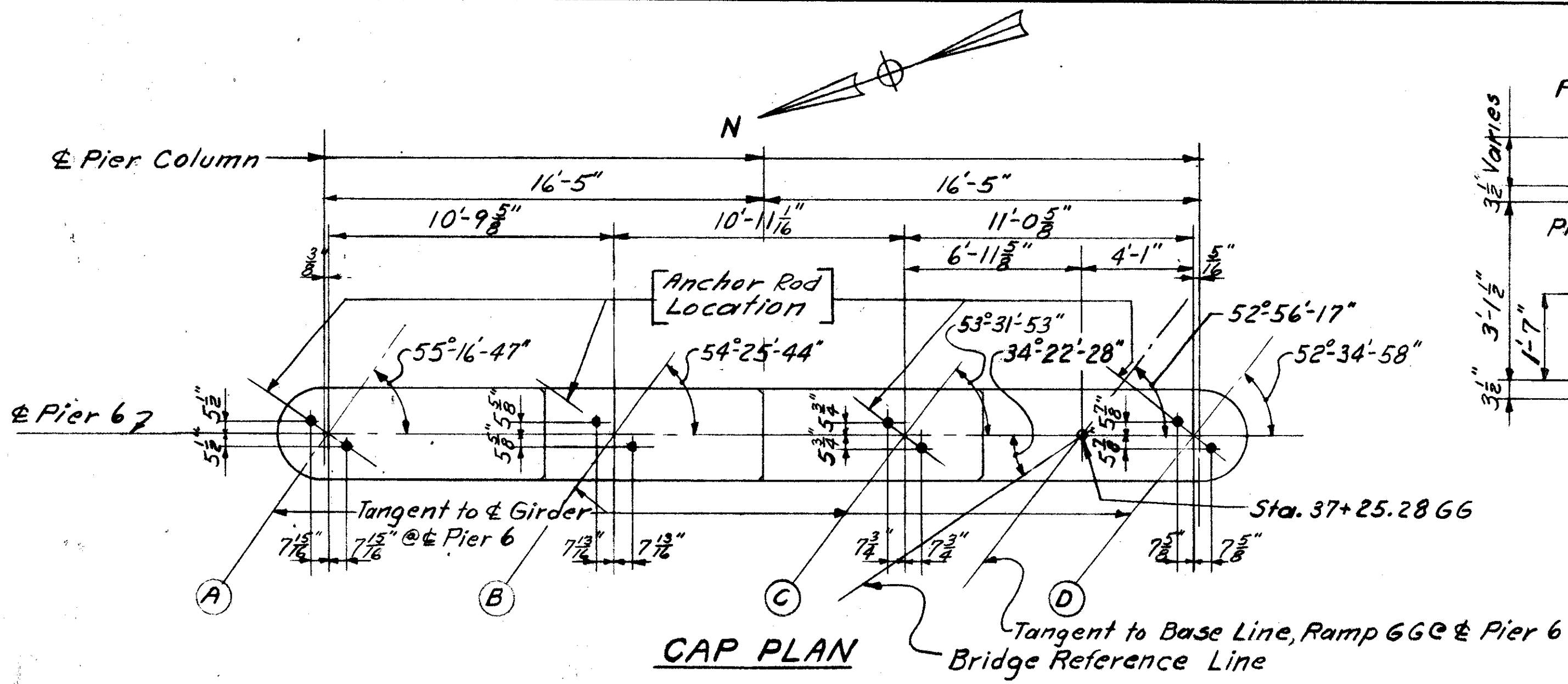
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CINCINNATI, OHIO

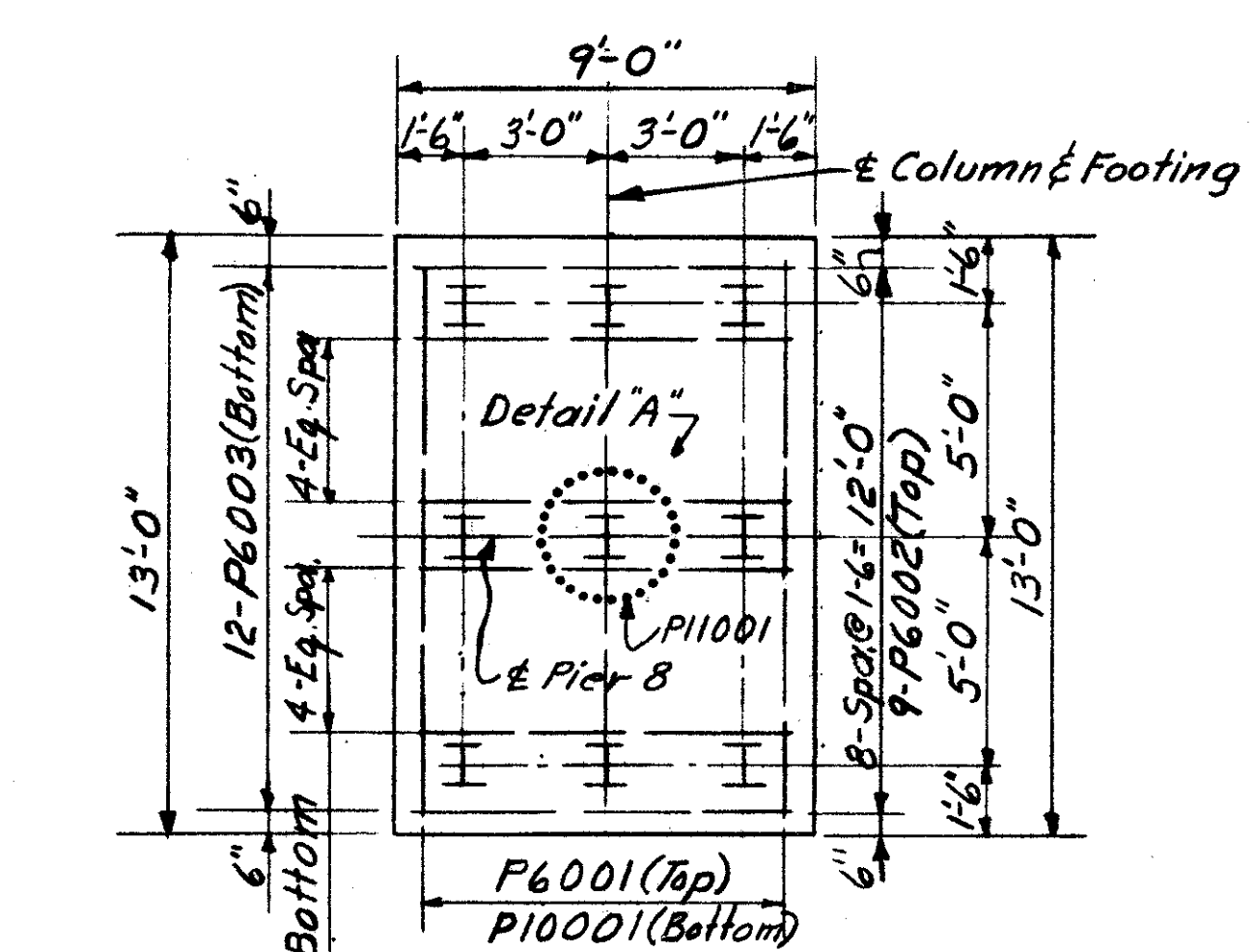
STAKE OUT PLAN
BRIDGE NO. HAM-71-0184

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	AKC		AMP	JWO 5/20/65	

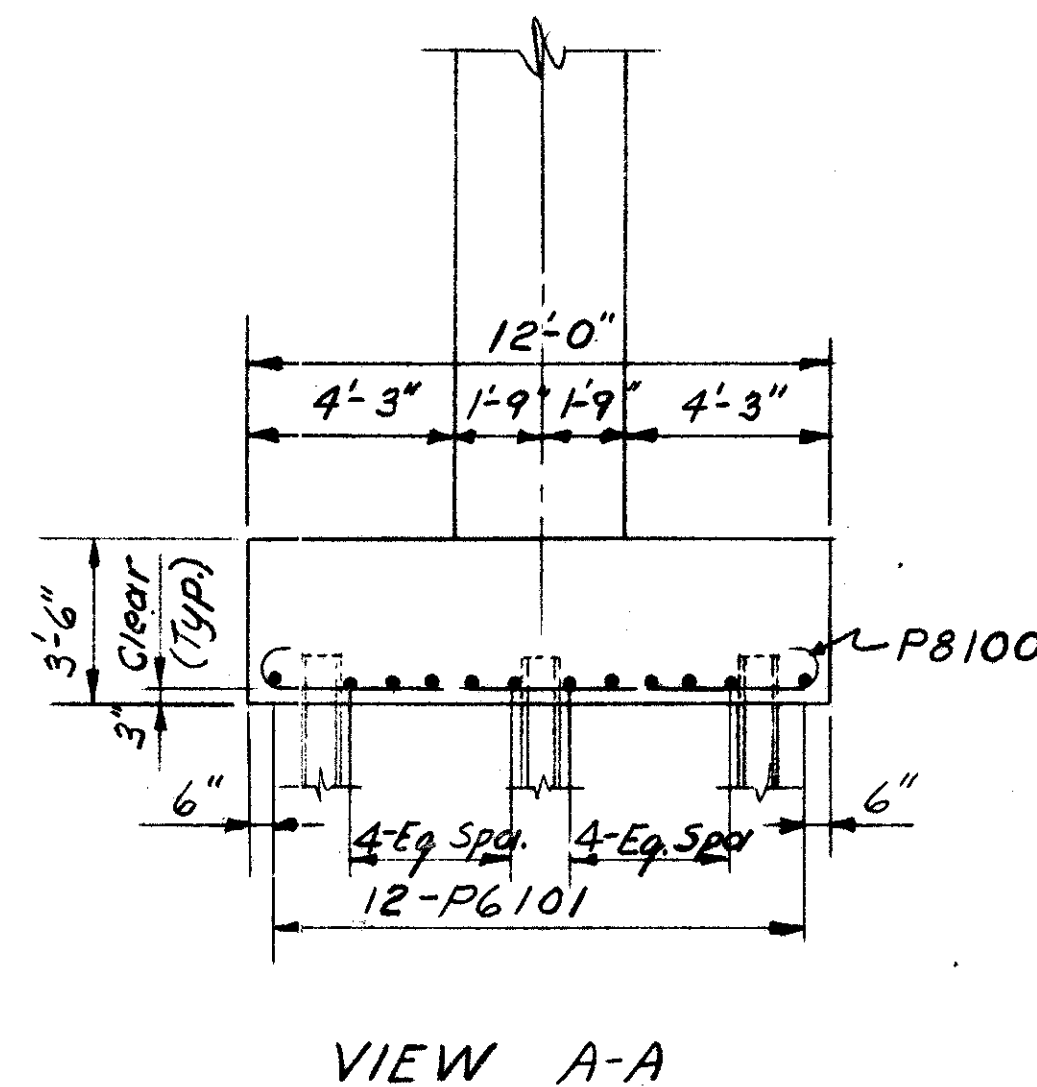
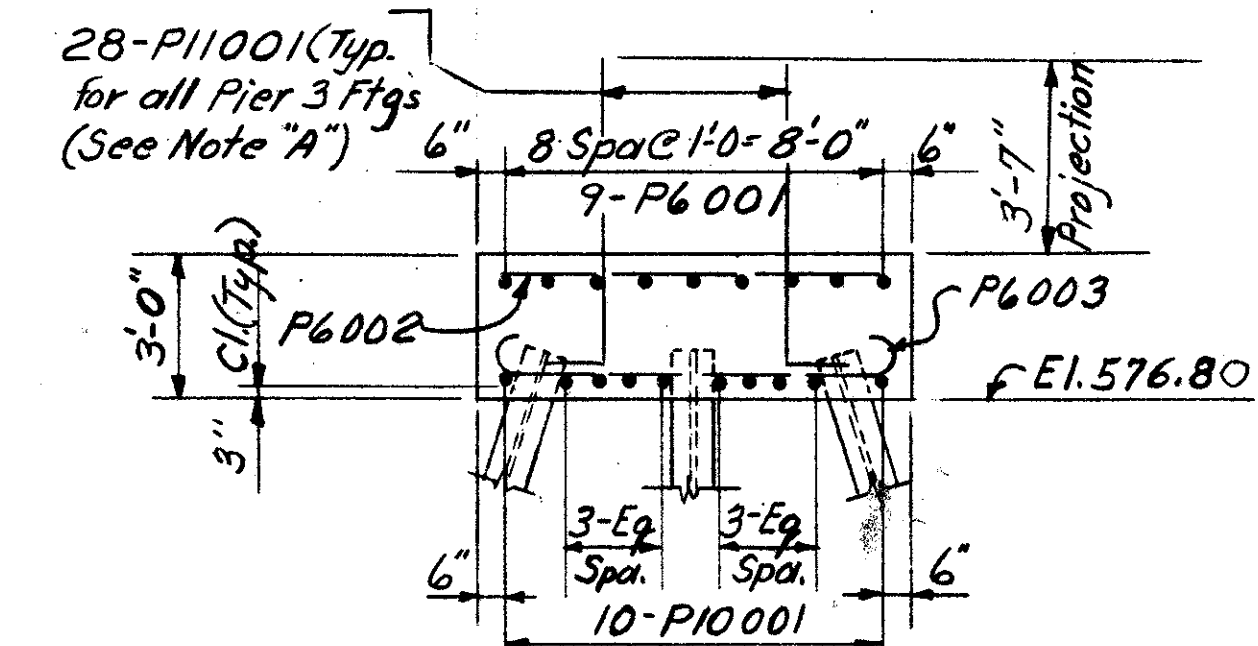
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



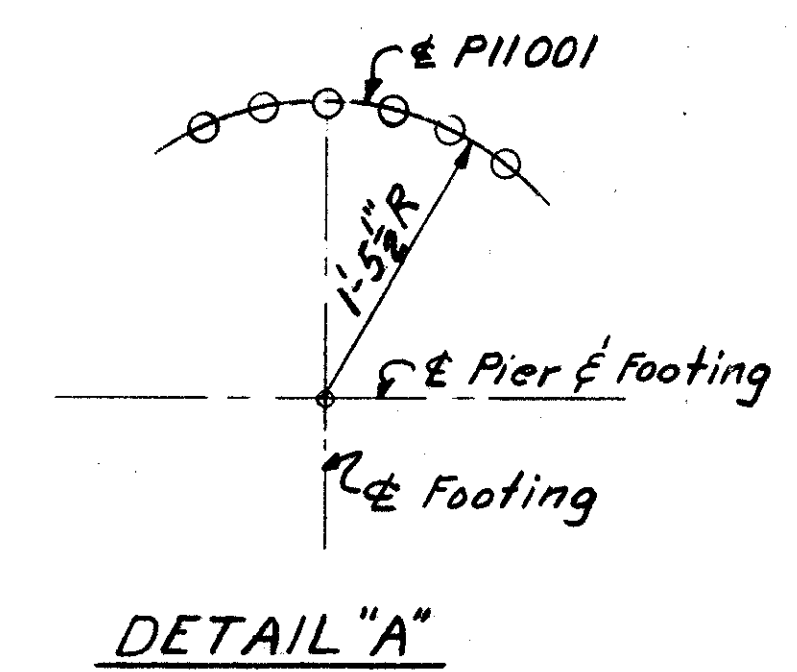
Note: Pile Batter not shown See Stake Out Plan.



Note: Pile Batter not shown See Stake Out Plan.



Note "A": The exposed portion of bars P11001 shall be given an asphaltic coating and wrapped with two layers of plastic film after the footing concrete has been poured. The plastic film shall be securely bound to the bars. This work shall be incidental to the concrete work.



Notes:
Special care shall be taken in placing reinforcing steel in the vicinity of bridge seat so as to avoid interference with the drilling of anchor rod holes. Concrete in footing shall be Class "E" and concrete in columns and caps shall be Class "C"

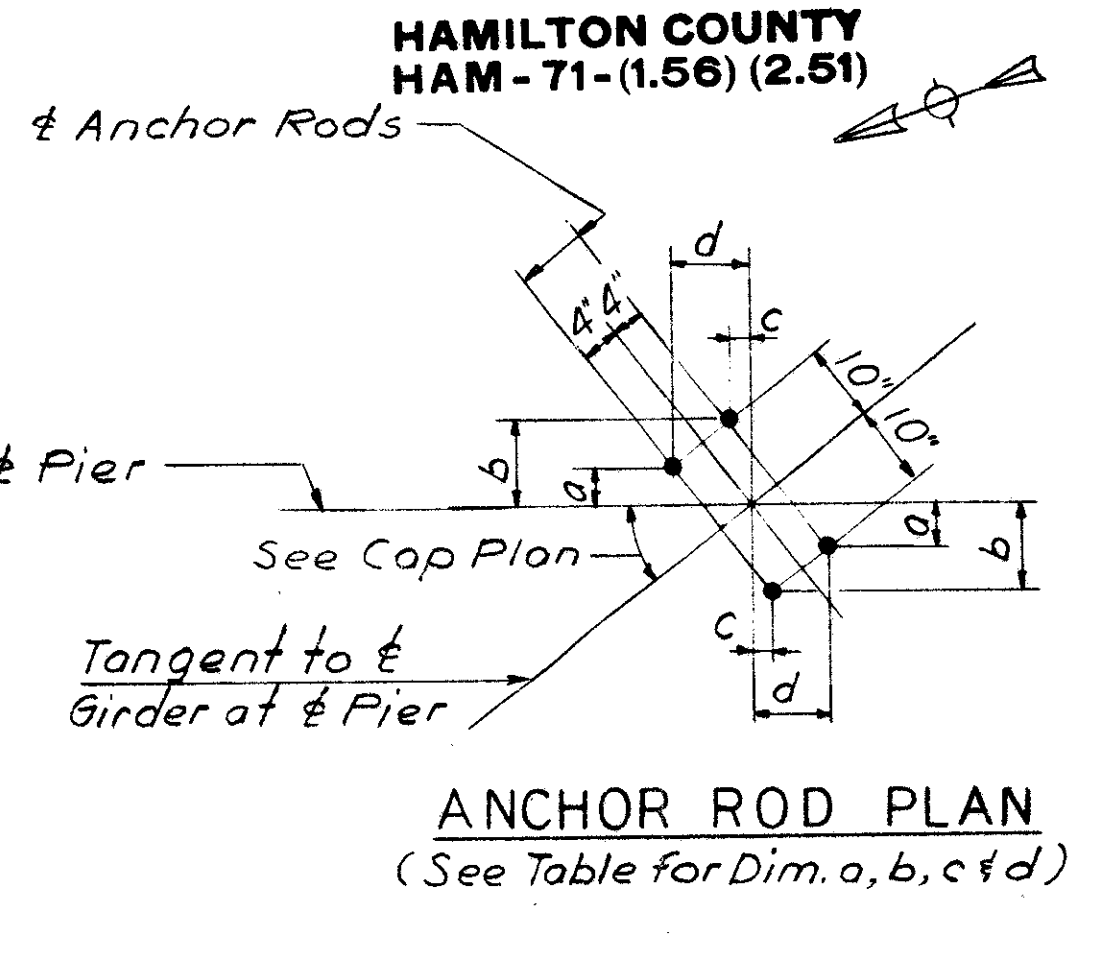
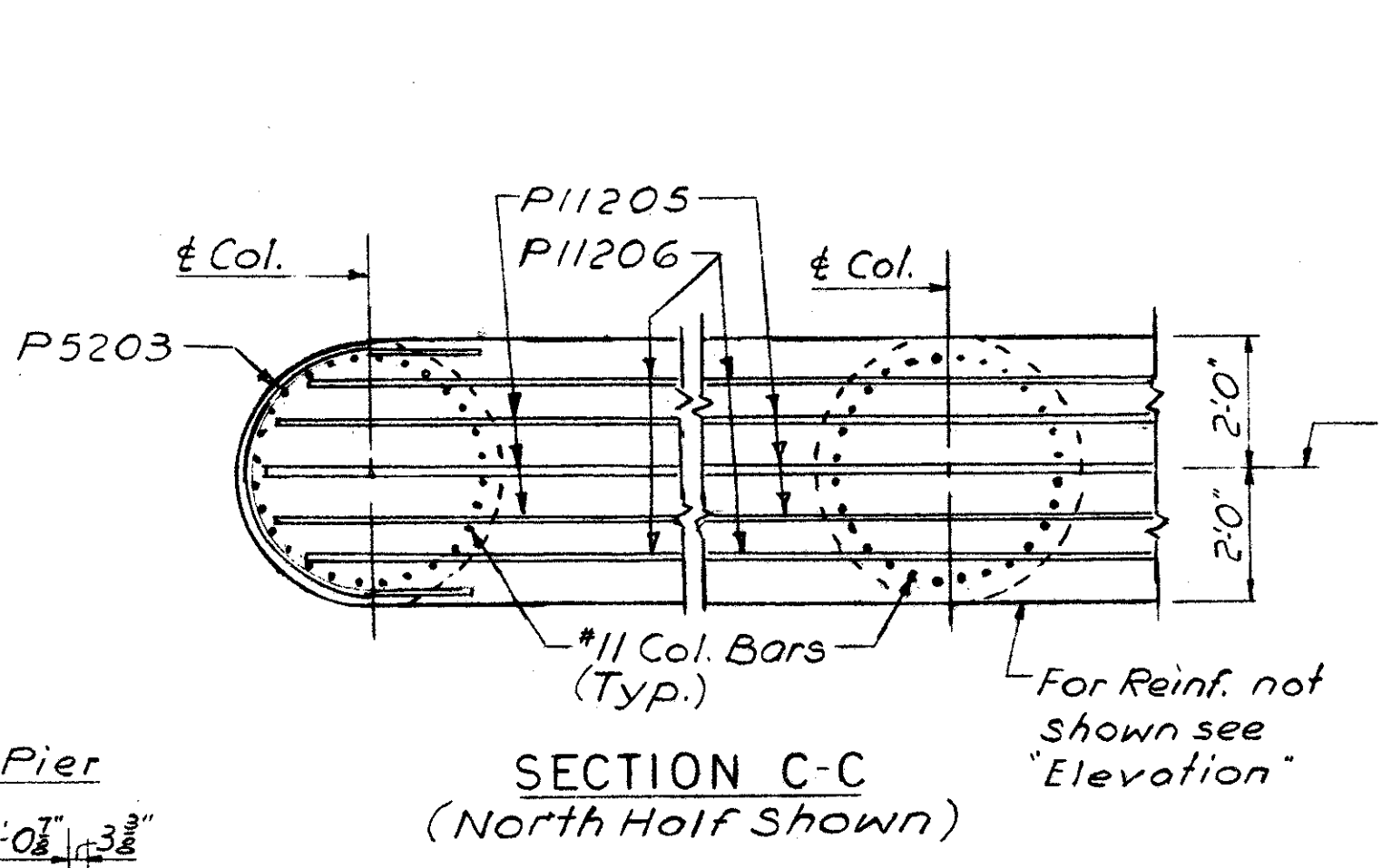
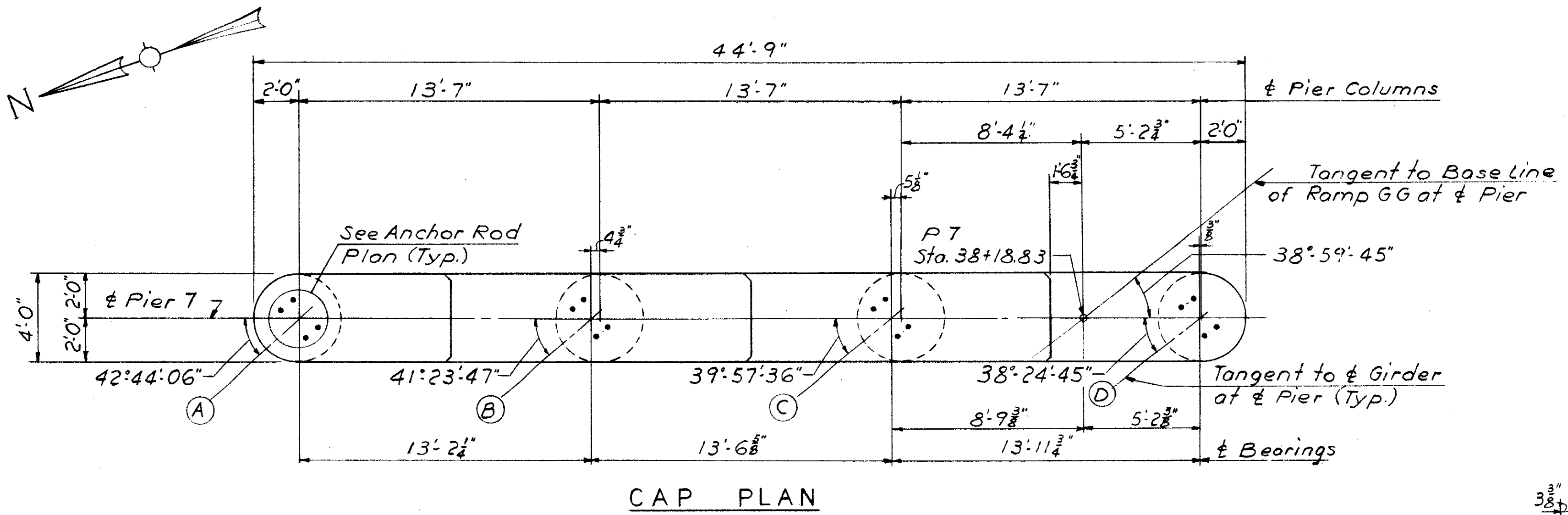
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CINCINNATI, OHIO

PIER 6 & PIER 3 FOOTING
BRIDGE NO. HAM - 71 - 0184

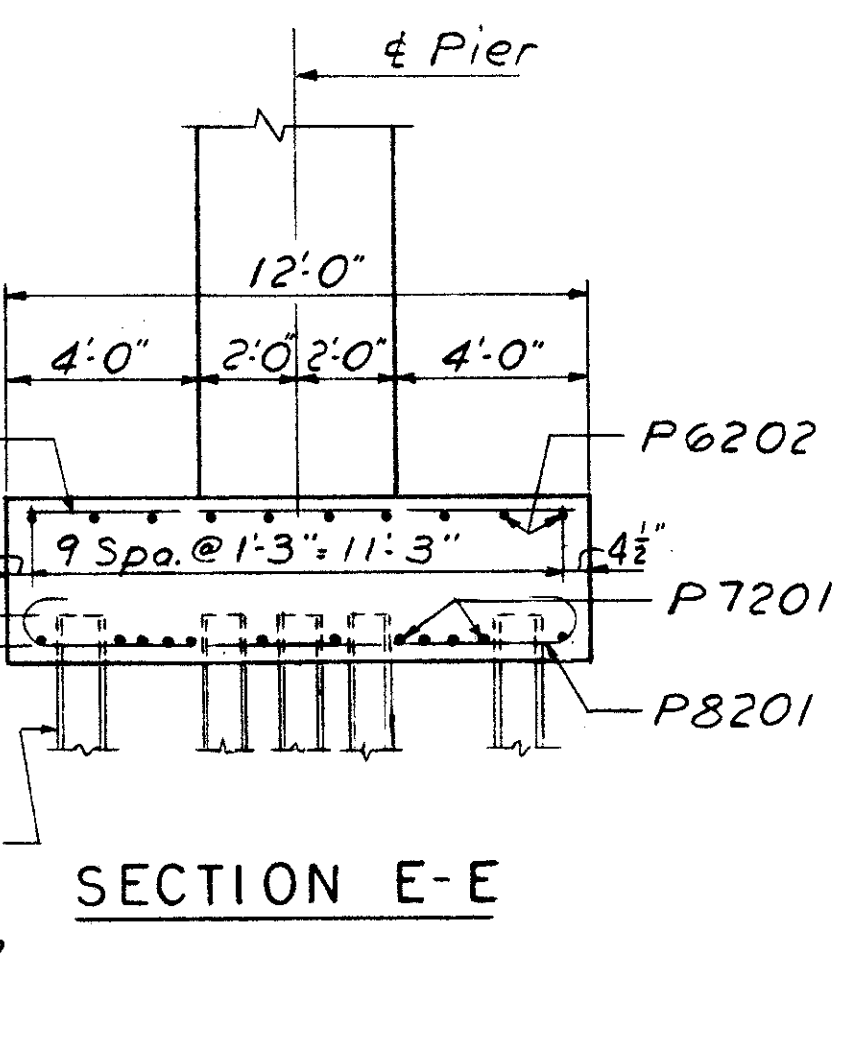
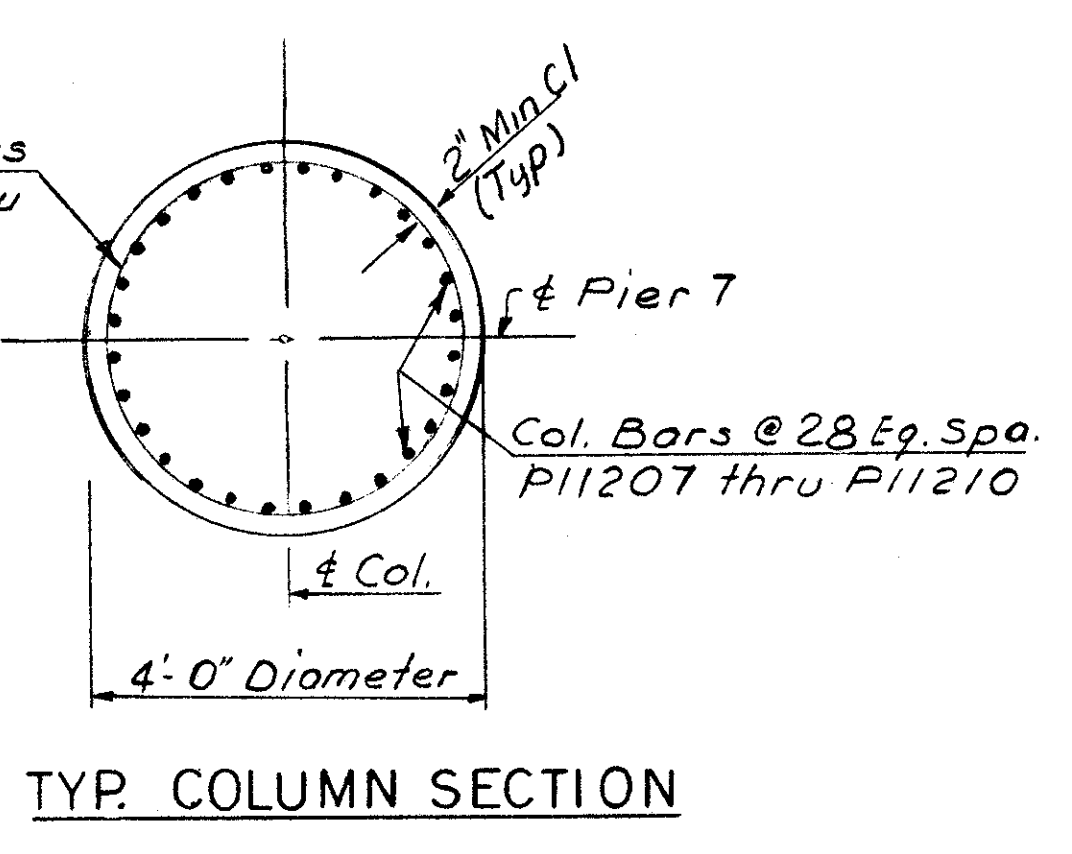
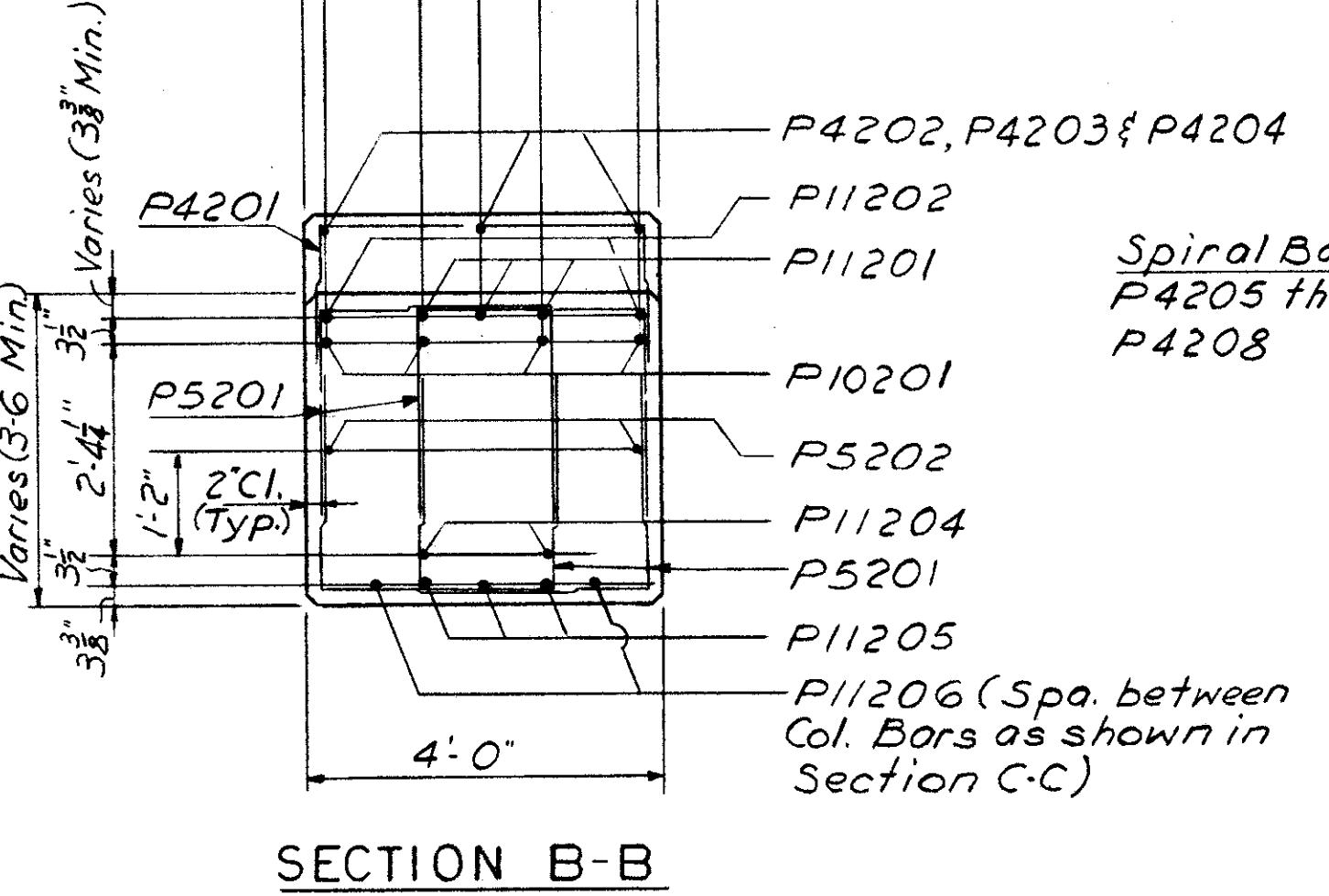
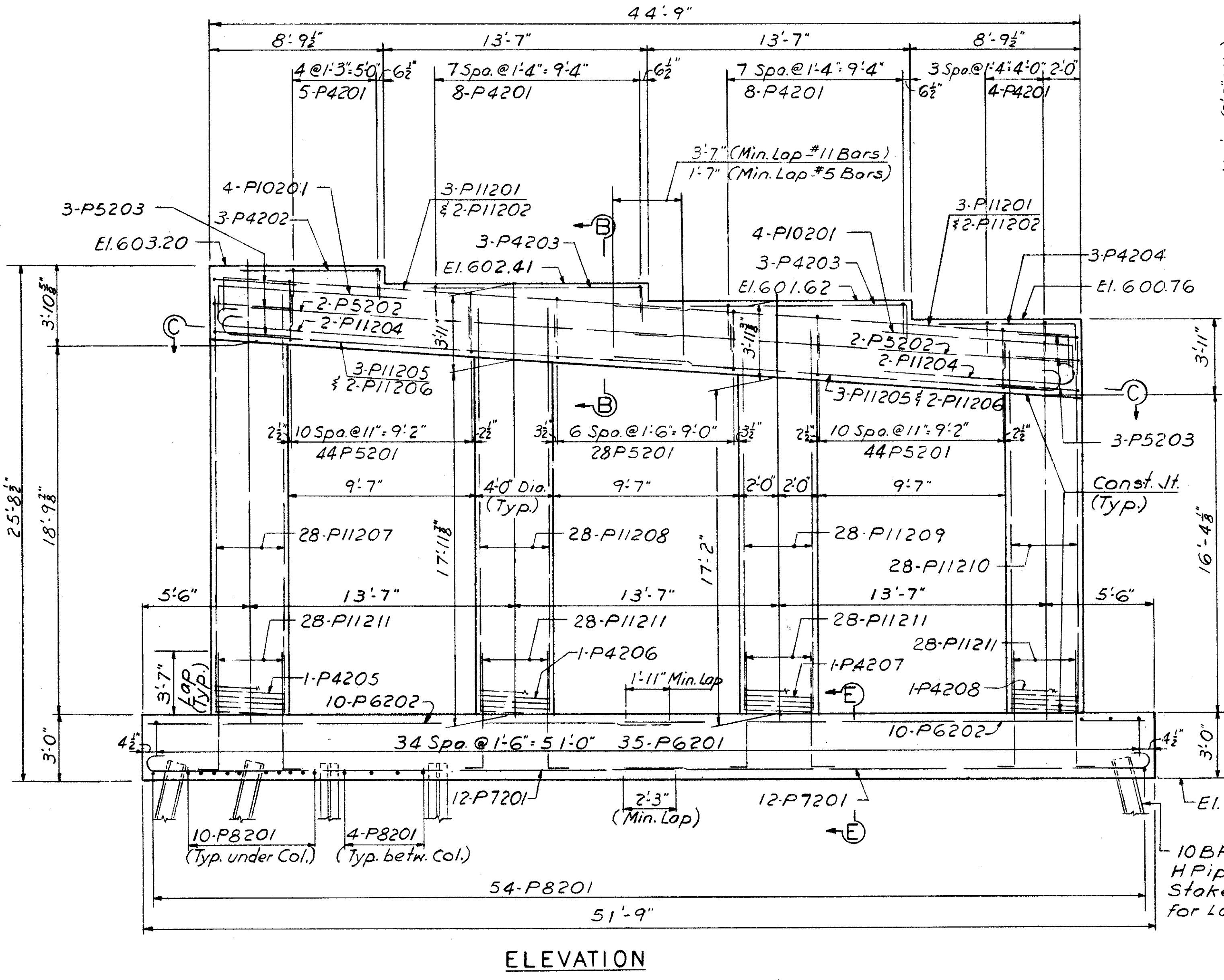
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	H.W.T.			JHO 5/20/65	

HAMILTON COUNTY
HAM-71-(1.56) (2.51)



ANCHOR ROD PLAN DIMENSIONS

Girder	a	b	c	d
A	4 3/8"	10 1/8"	3 3/8"	9 3/4"
B	4 3/8"	10 1/8"	3 5/8"	9 5/8"
C	5 3/8"	10 1/4"	3 3/8"	9 1/2"
D	5 3/8"	10 5/8"	3 1/8"	9 3/8"



Notes:
Special care shall be taken in placing reinforcing steel in the vicinity of bridge seat so as to avoid interference with the drilling of anchor rod holes.
Concrete in footing shall be Class "E" and concrete in columns and cap shall be Class "C".

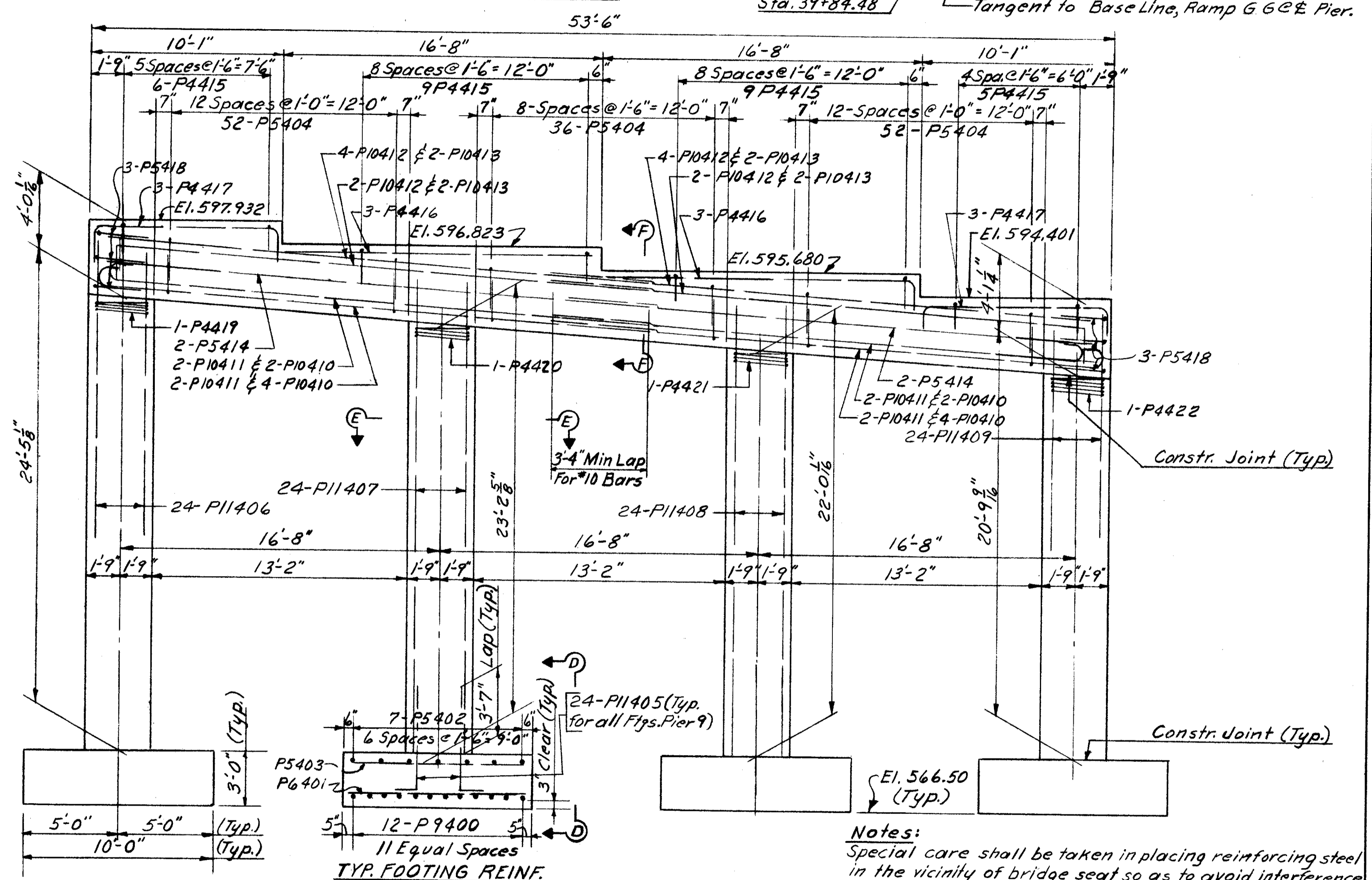
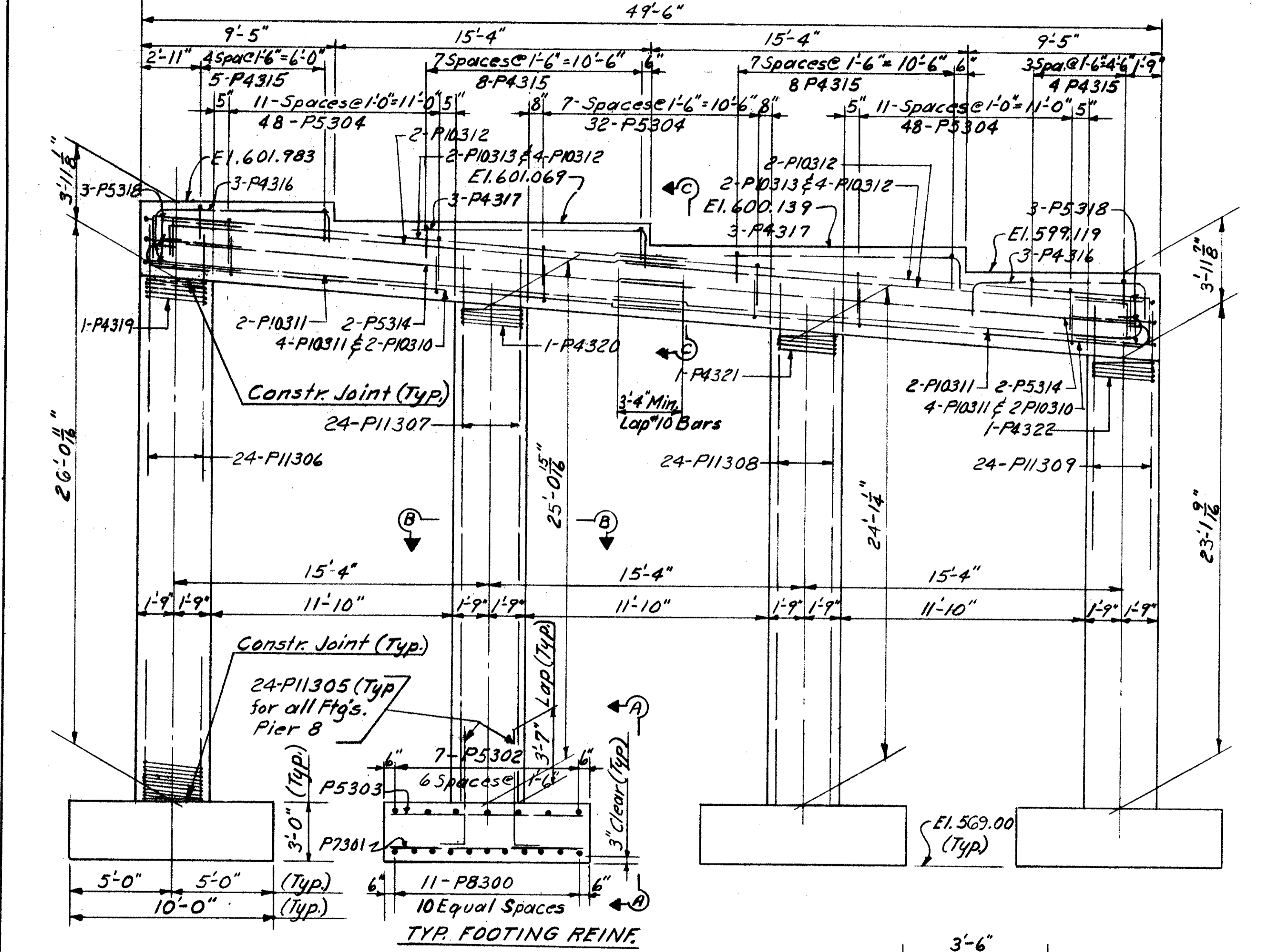
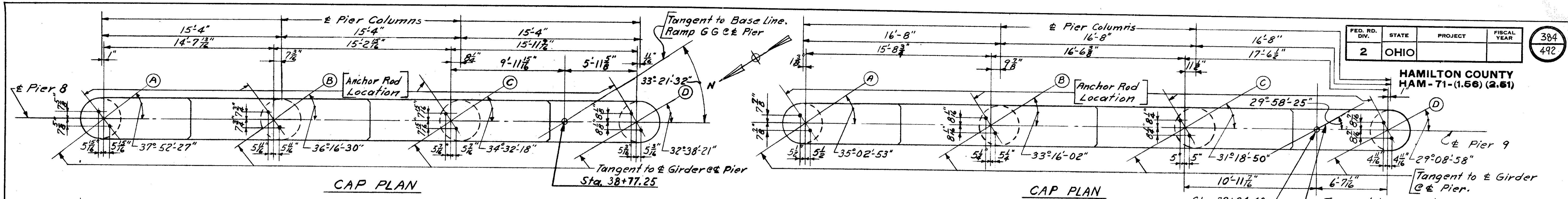
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PIER 7
BRIDGE NO. HAM-71-0184

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	S.U.		R.L.K. 4-15-65	J.H. 5/20/65	

HAMILTON COUNTY
HAM - 71 - (1.56) (2.61)



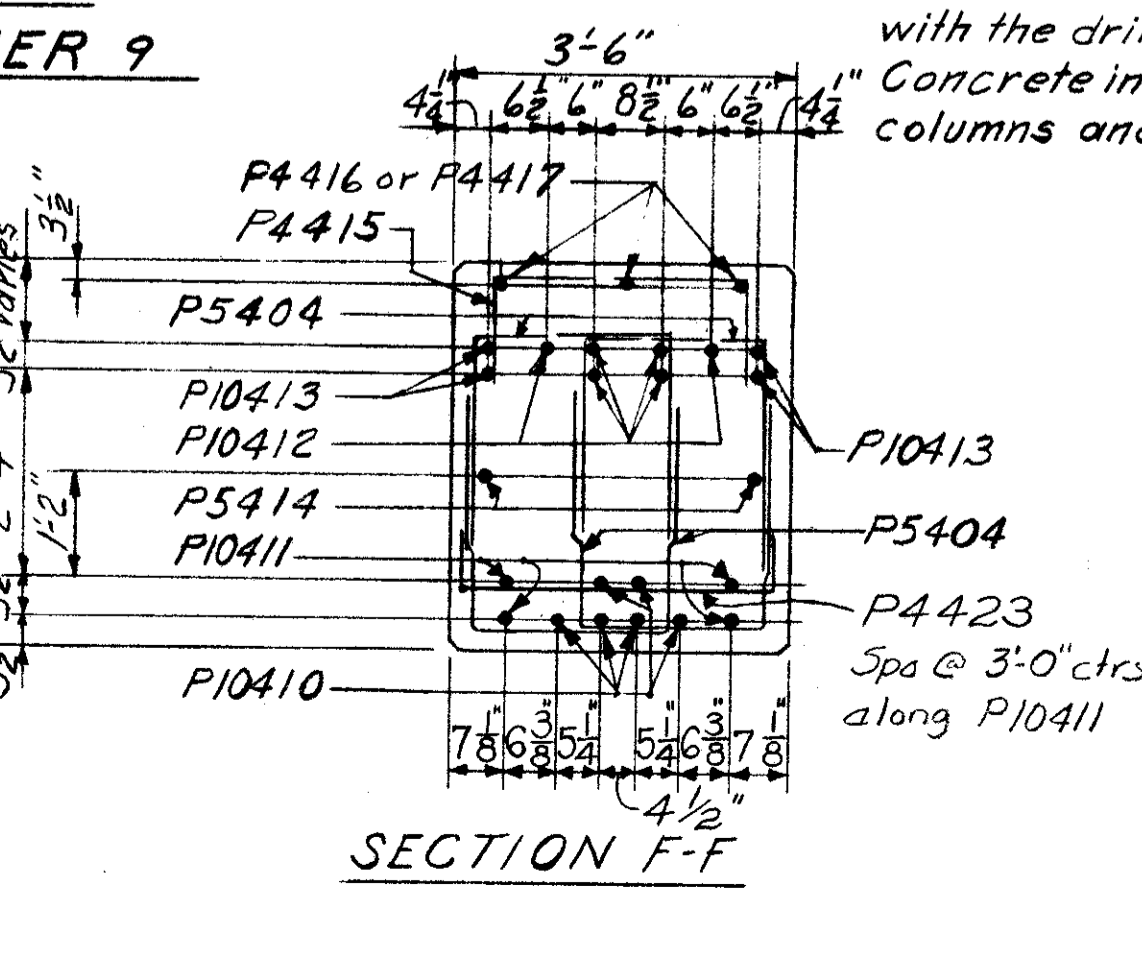
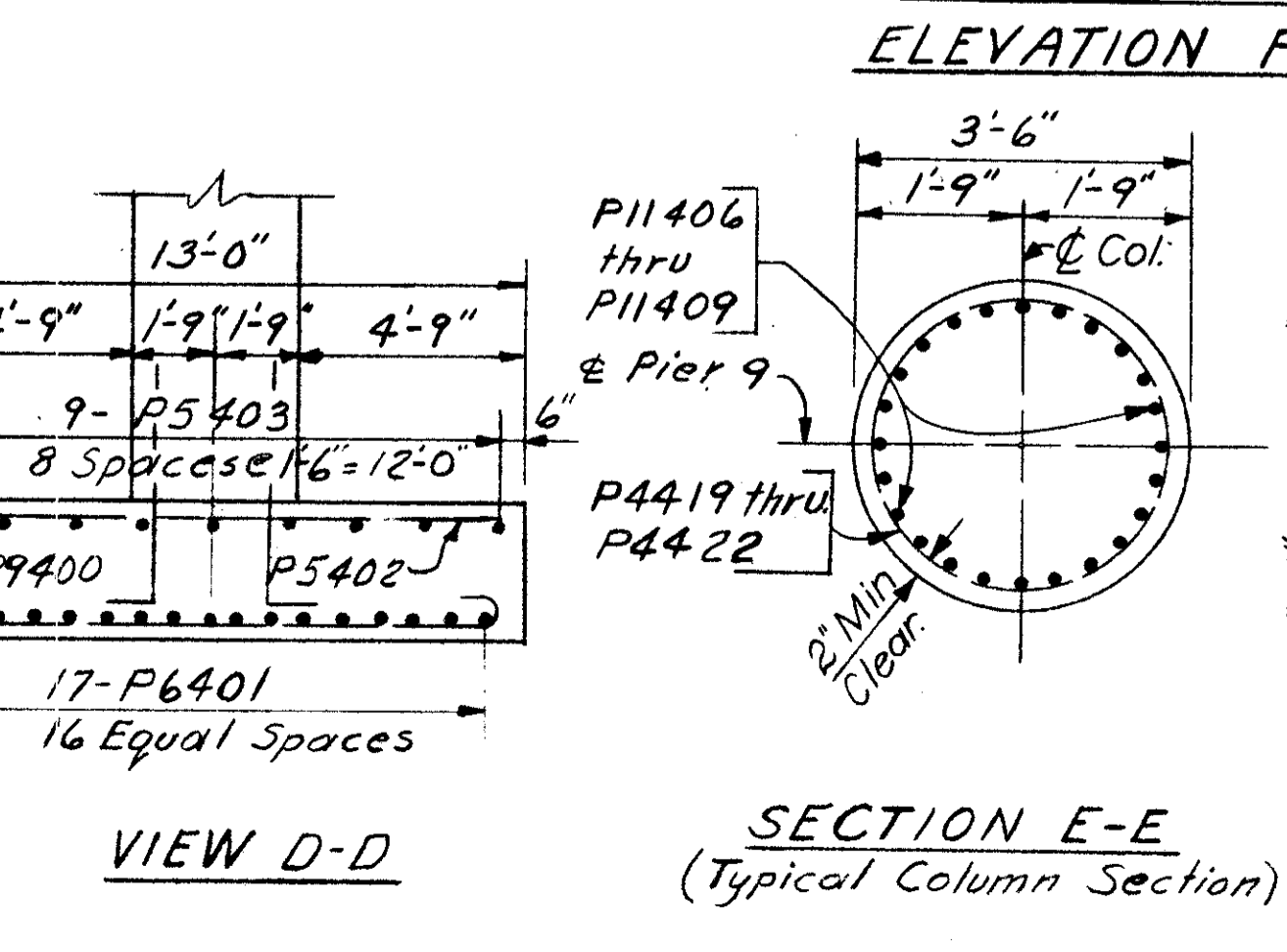
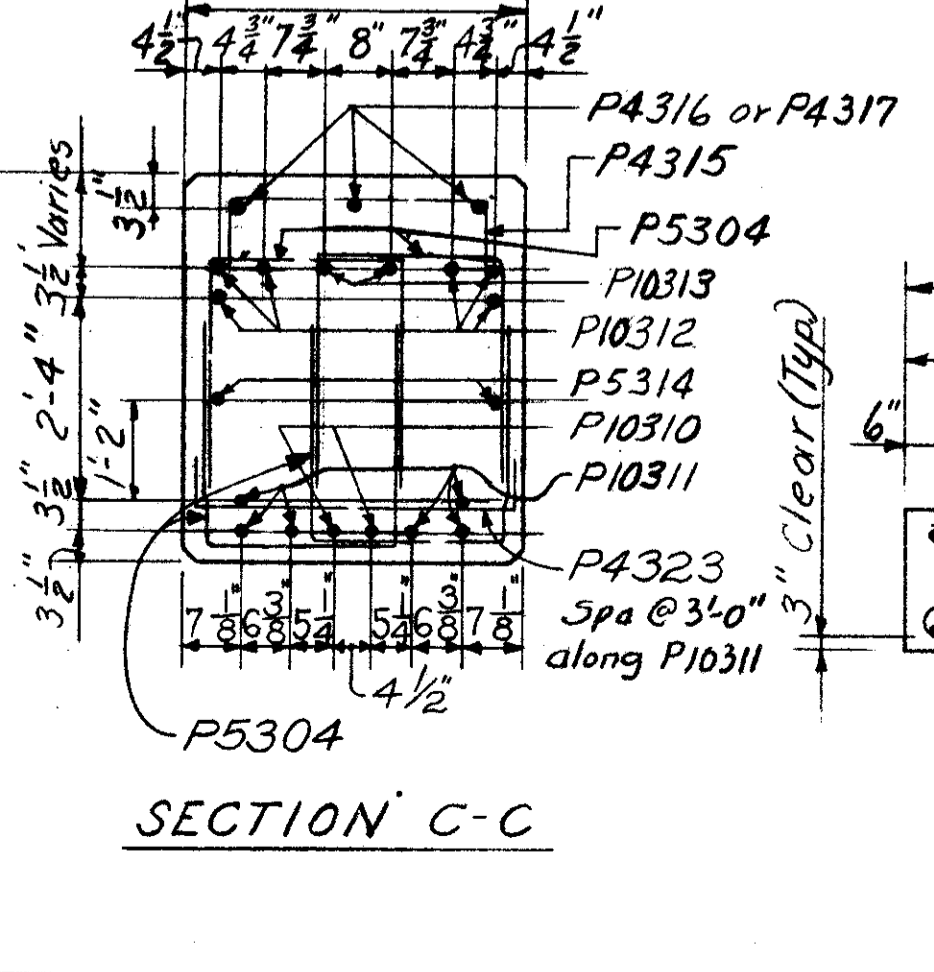
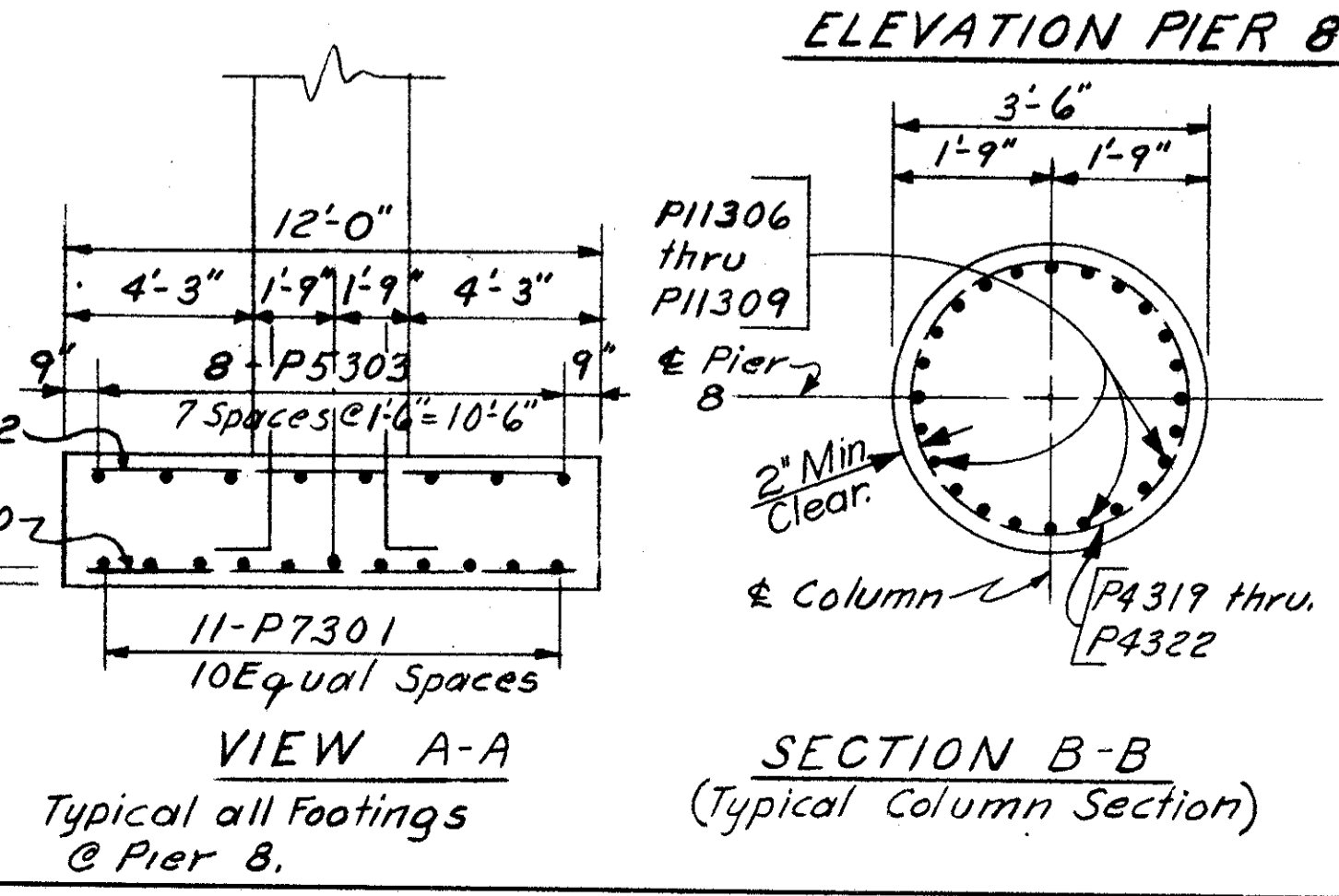
Notes:
Special care shall be taken in placing reinforcing steel in the vicinity of bridge seat so as to avoid interference with the drilling of anchor rod holes.
Concrete in footing shall be Class "E" and concrete in columns and caps shall be Class "C".

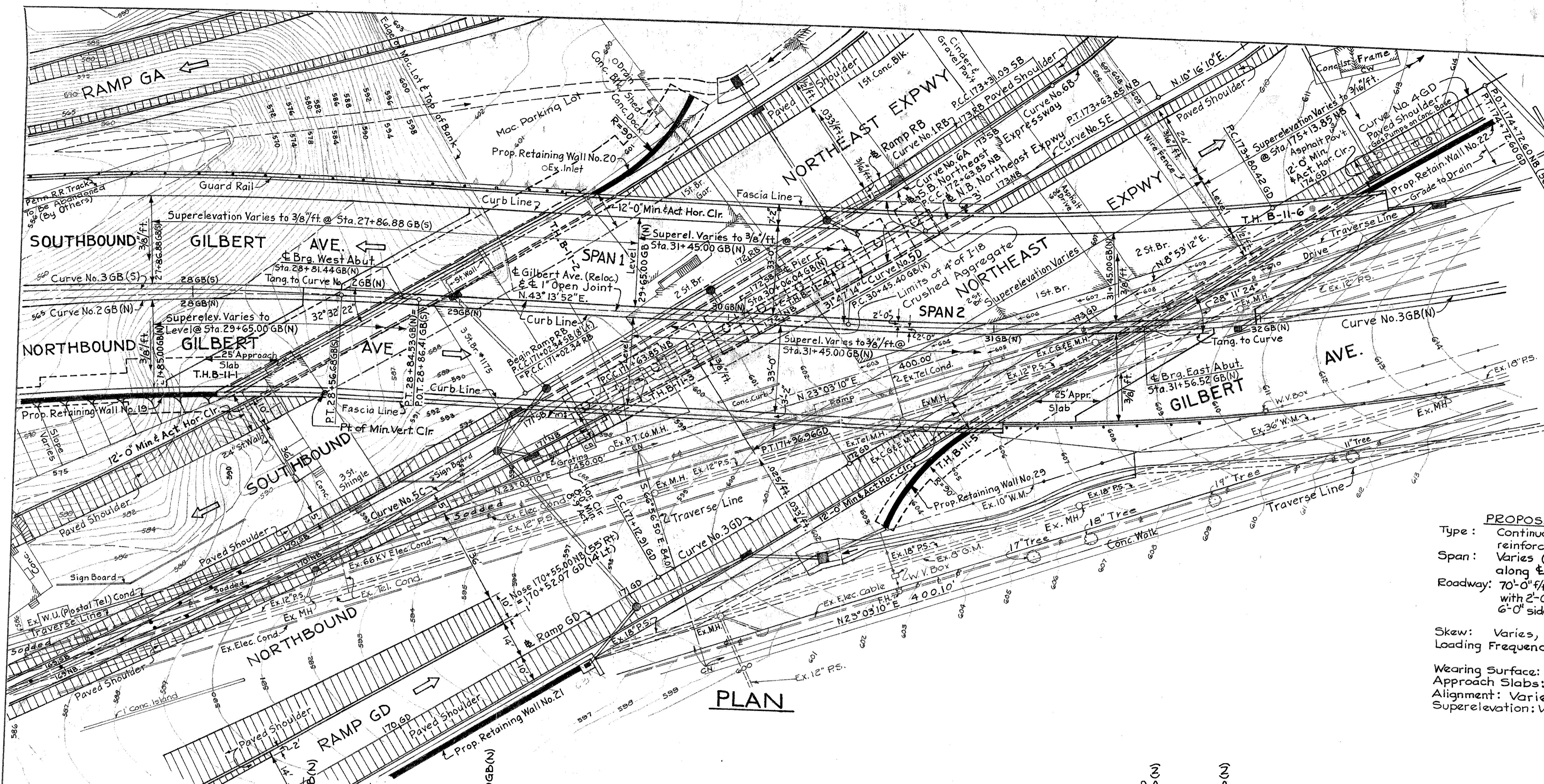
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FEB 20 1985

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PIERS 8 & 9
BRIDGE NO. HAM - 71 - 0184

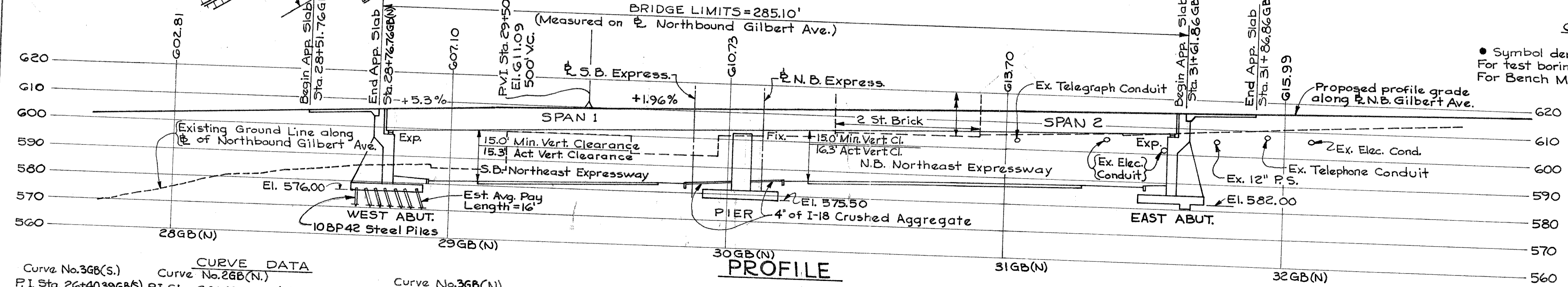
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	H.W.T.			J40 5/20/65	





PLAN

PROPOSED STRUCTURE
 Type: Continuous steel plate girders with reinforced concrete deck and substructure.
 Span: Varies (124'6", 150'48" 1/2 brgs; measured along E Gilbert Ave. (Relocated))
 Roadway: 70'-0" 1/2 curbs including 4'-0" raised median with 2'-0" safety curb Northbound Rdwy. & 6'-0" sidewalk Southbound Rdwy.
 Skew: Varies, see Plan
 Loading Frequency: CF = 2000 (57)
 Wearing Surface: 1" Monolithic Concrete
 Approach Slabs: AS-1-54 (25'-0" Long)
 Alignment: Varies, see Plan
 Superelevation: Varies, see Plan



PROFILE

GENERAL NOTES
 • Symbol denotes drill hole
 For test boring data, see Sheet 7 & 8 of 13
 For Bench Marks, see Sheet 40

CURVE DATA

Curve No. 3GB(S)	Curve No. 2GB(N)	Curve No. 3GB(N)
P.I. Sta. 26+40.39GB(S)	P.I. Sta. 26+68.23GB(N)	P.I. Sta. 32+32.38GB(N)
Δ = 19° 39' 42"	Δ = 19° 39' 42"	Δ = 20° 21' 02"
D _c = 4° 30' 00"	D _c = 4° 30' 00"	D _c = 5° 30' 00"
R = 1,273.24'	R = 1,273.24'	R = 1,041.74'
L = 436.93'	L = 436.93'	L = 370.01'
T = 220.63'	T = 220.63'	T = 186.97'

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SITE PLAN
 BRIDGE No. HAM-71-0196
 GILBERT AVENUE

H & E BRIDGE No. 11

Traffic Count: 1986 ADT = 8800
DHV = 880

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED	DATE
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ESTIMATED QUANTITIES

Item	Total	Unit	Description	Super-structure	East Abutment	West Abutment	Pier	General
E-2	Lump	Sum	Cofferdams, cribs & sheeting					Lump
E-2	4775	Cu.Yds.	Unclassified excavation		2440	1650	685	
E-2	400	Cu.Yds.	Rock (or shale) excavation		270		130	
S-1	782	Cu.Yds.	Class "C" concrete, superstructure	782				
S-1	187	Cu.Yds.	Class "C" concrete, piers above footings				187	
S-1	1044	Cu.Yds.	Class "E" concrete, abutments above footings		5844	4596		
S-1	1050	Cu.Yds.	Class "E" concrete, Footings		435	419	196	
S-3	56	Lin. Ft.	Waterproofing, premolded sealing strip		29	27		
S-4	460,791	Lbs.	Reinforcing Steel	200741	97844	86437	75769	
* S-7	1243000	Lbs.	Structural Steel	1243000				
* S-8	1243000	Lbs.	Field painting of structural steel	1243000				
S-9	113	Sq. Ft.	1" preformed expansion joint filler (M10.02 Type 1)		59	54		
S-14	595	Lin. Ft.	Railing type "C" (aluminum rail & supports, concrete parapet & end posts)	557	20	18		
S-16	Lump	Sum	First test pile					Lump
S-18	3392	Lin. Ft.	Steel piles, 10 BP 42			3392		
S-25			Electric Lighting System (See Sh. No 204)					
S-29	640	Cu.Yds.	Porous backfill		360	280		
S-29	Lump	Sum	Drain inlet including supports and horizontal collector system	Lump				
S-29	58	Lin. Ft.	8" standard pipe downspout, wrought iron or hot-dipped galvanized steel, including specials				58	
S-29	108	Lin. Ft.	8" bituminous coated helical perforated corrugated metal pipe [Sec. M-6.4(h-c)], (including specials)		60	48		
S-29	348	Lin. Ft.	8" bituminous coated helical perforated corrugated metal pipe [Sec. M-6.4(h-c)], (including specials and sand)		186	162		
S-29	26	Lin. Ft.	12" reinforced concrete sewer pipe [Sec. M-6.6(a)]		10	16		
S-29	48	Lin. Ft.	8" bituminous coated helical corrugated metal pipe [Sec. M-6.4(h-c)], (including specials)			48		
S-101	782	Each	Water-reducing set retarding admixture	782				

GENERAL NOTES

STEEL PILES: shall be driven with a hammer of not less than 11,000 ft. lbs. per blow to firm contact with rock or shale. If the length of penetration is approximately equal to the depth of rock and shale according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Section S-18.05 is not less than the following value for a pile hammer of the indicated energy rating.

60 tons per pile for west abutment using a 11,000 ft. lb. hammer or 55 tons per pile using a 15,000 ft. lb. hammer. If the energy rating of the hammer is between the rating as shown above, the required formula-capacity shall be determined by interpolation. The design of the west abutment is based on 35 tons per pile.

DESIGN NOTES:

Design Loading CF = 2000 (57)
 Concrete Class C - Basic unit stress 1333 psi
 Concrete Class E - Basic unit stress 1133 psi
 Structural Steel - ASTM A36 - Basic unit stress 20,000 psi (ASTM A7 and A373 steel not permitted except for piling)
 Reinforcing Steel - ASTM A15, A16, A160. Deformed Intermediate or Hard Grade, Basic unit stress 20,000 psi. Except spiral reinforcement may be plain structural grade with basic unit stress of 18,000 psi.

FOUNDATION BEARING PRESSURE: the east abutment and pier footings are designed for a maximum bearing pressure of 3.0 tons per sq. ft.

FOOTINGS: for the east abutment and the pier shall extend a minimum of 3" into undisturbed rock or to the elevations shown on the plans, whichever is lower.

MACHINE FINISH OF CONCRETE: at the contractor's option, the concrete deck may be finished by the use of a finishing machine.

ADDITIONAL NOTES: For additional notes see Notes 1 thru 8, General Notes, Typical Drawing No. 443

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

387
492

HAMILTON COUNTY
HAM - 71-(1.56) (2.51)

* ITEMS CHARGEABLE TO CINCINNATI AND SUBURBAN BELL TELEPHONE CO.

Item	Unit	Description	Amount
S-7	Lbs.	Structural Steel	3600
S-8	Lbs.	Field Painting of Structural Steel	3600

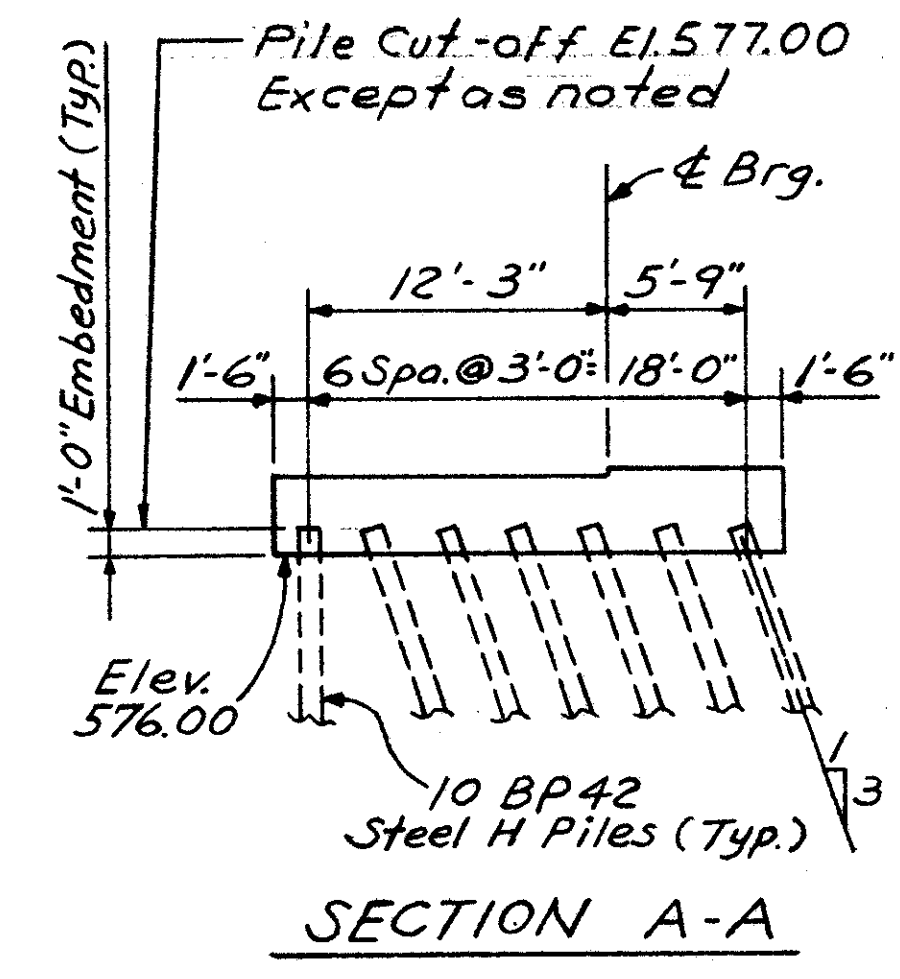
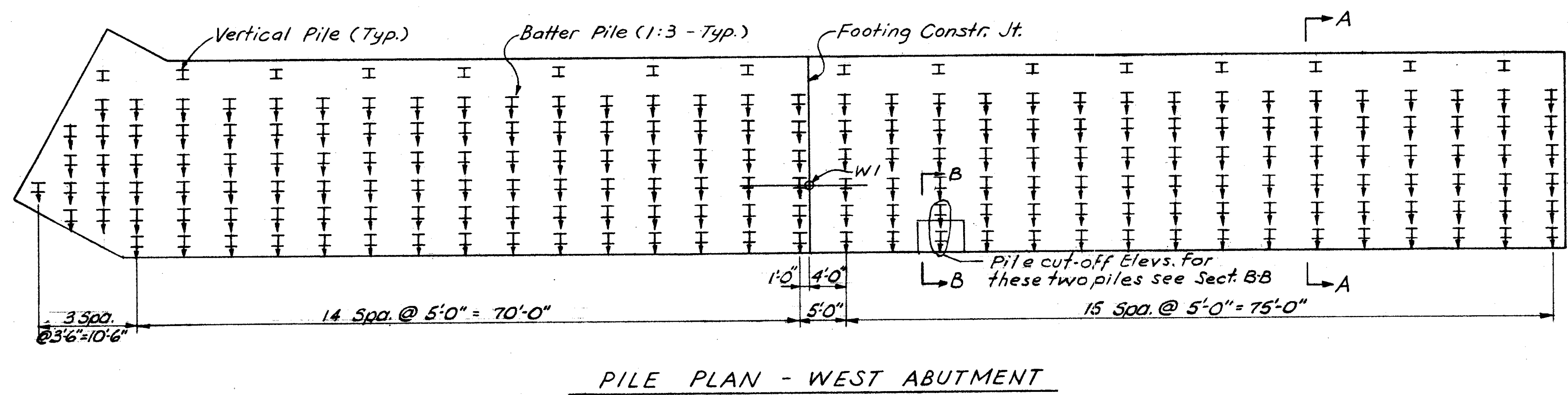
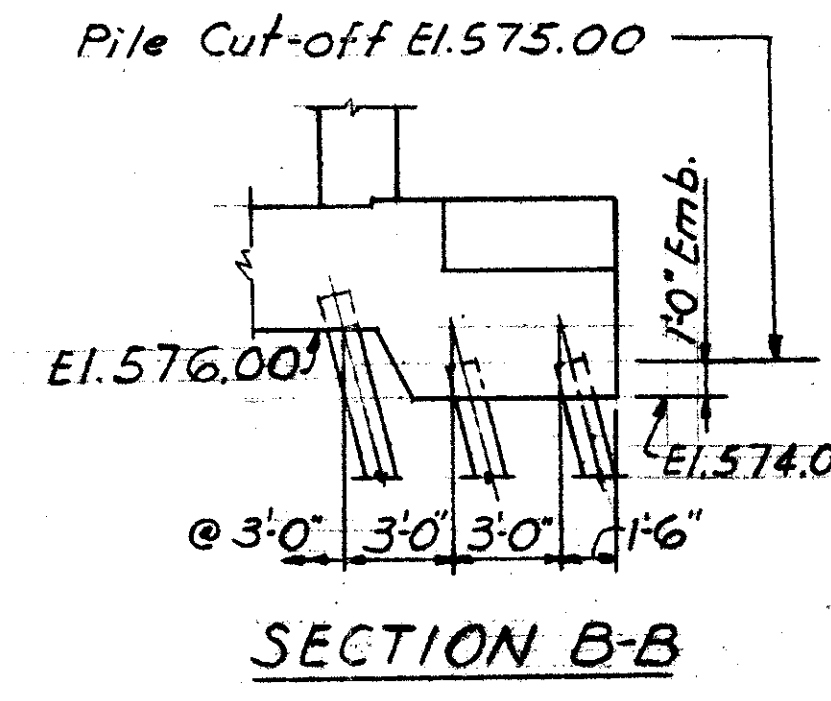
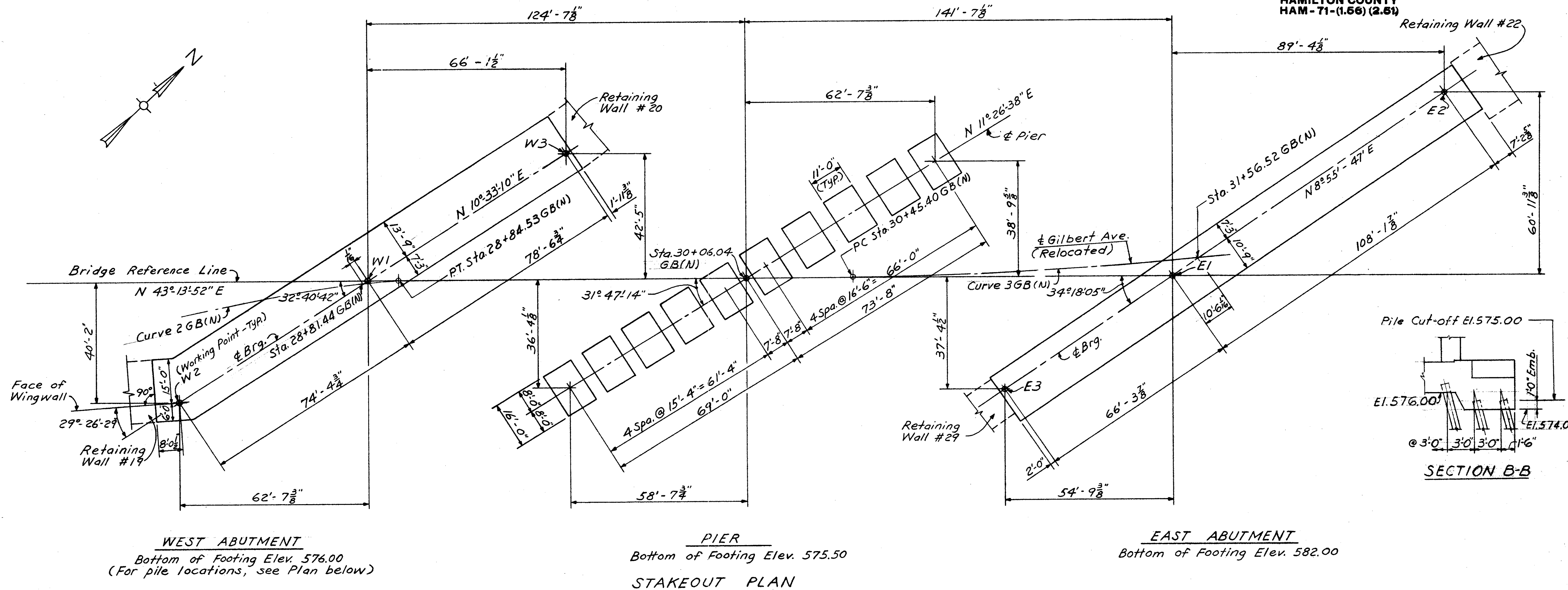
* ITEMS CHARGEABLE TO CINCINNATI GAS AND ELECTRIC CO.

Item	Unit	Description	Amount
S-7	Lbs.	Structural Steel	8550
S-8	Lbs.	Field Painting of Structural Steel	8550

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HAZLET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
ESTIMATED QUANTITIES & NOTES					
BRIDGE NO. HAM - 71-0196					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	S.U.		JJA	JH	
			3-17-65	5/20/65	

HAMILTON COUNTY
HAM - 71-(1.66) (2.51)



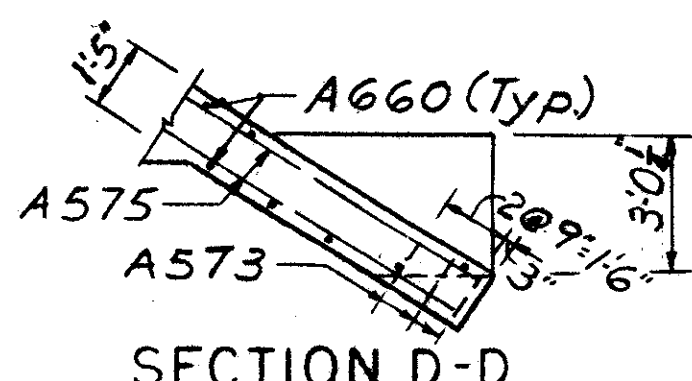
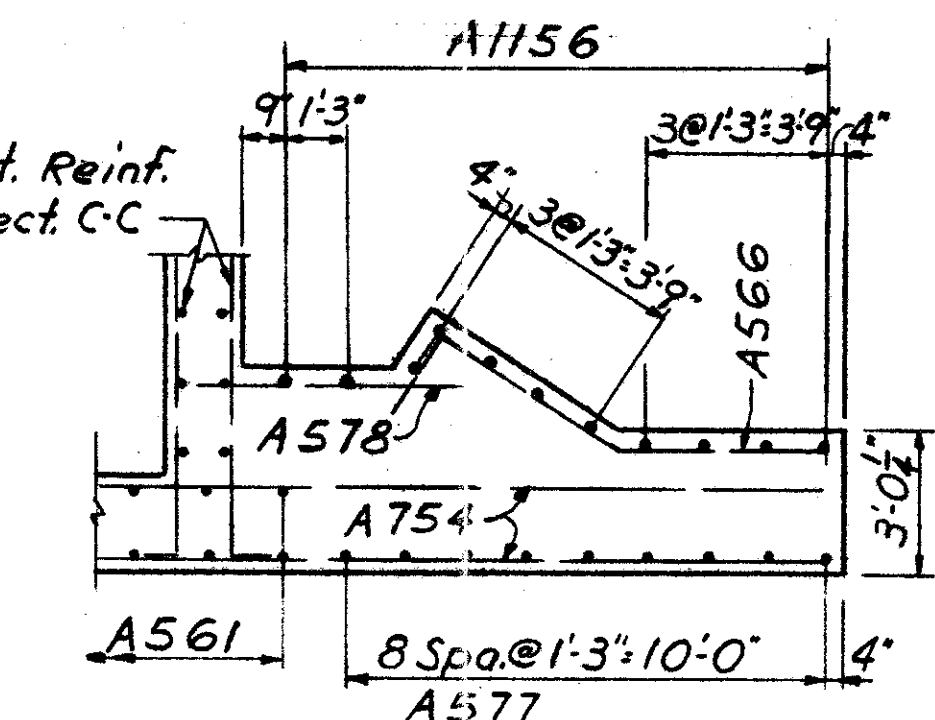
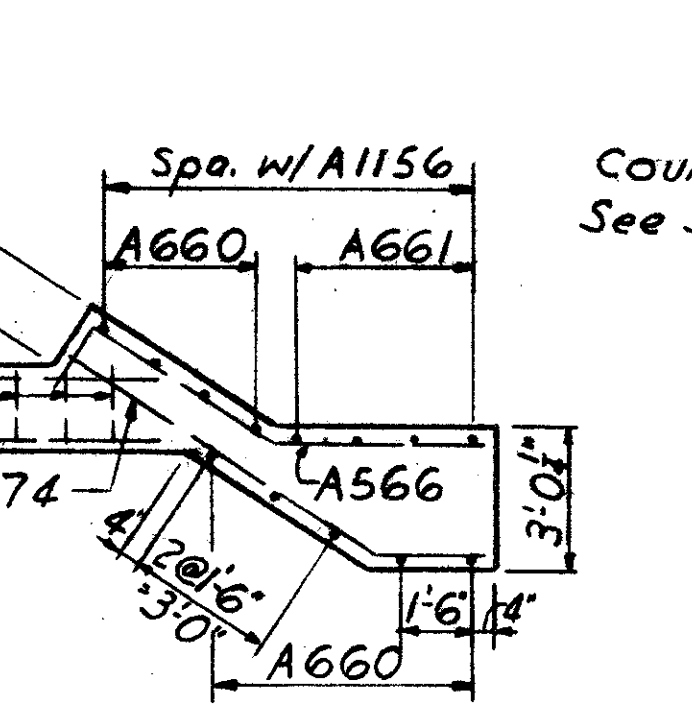
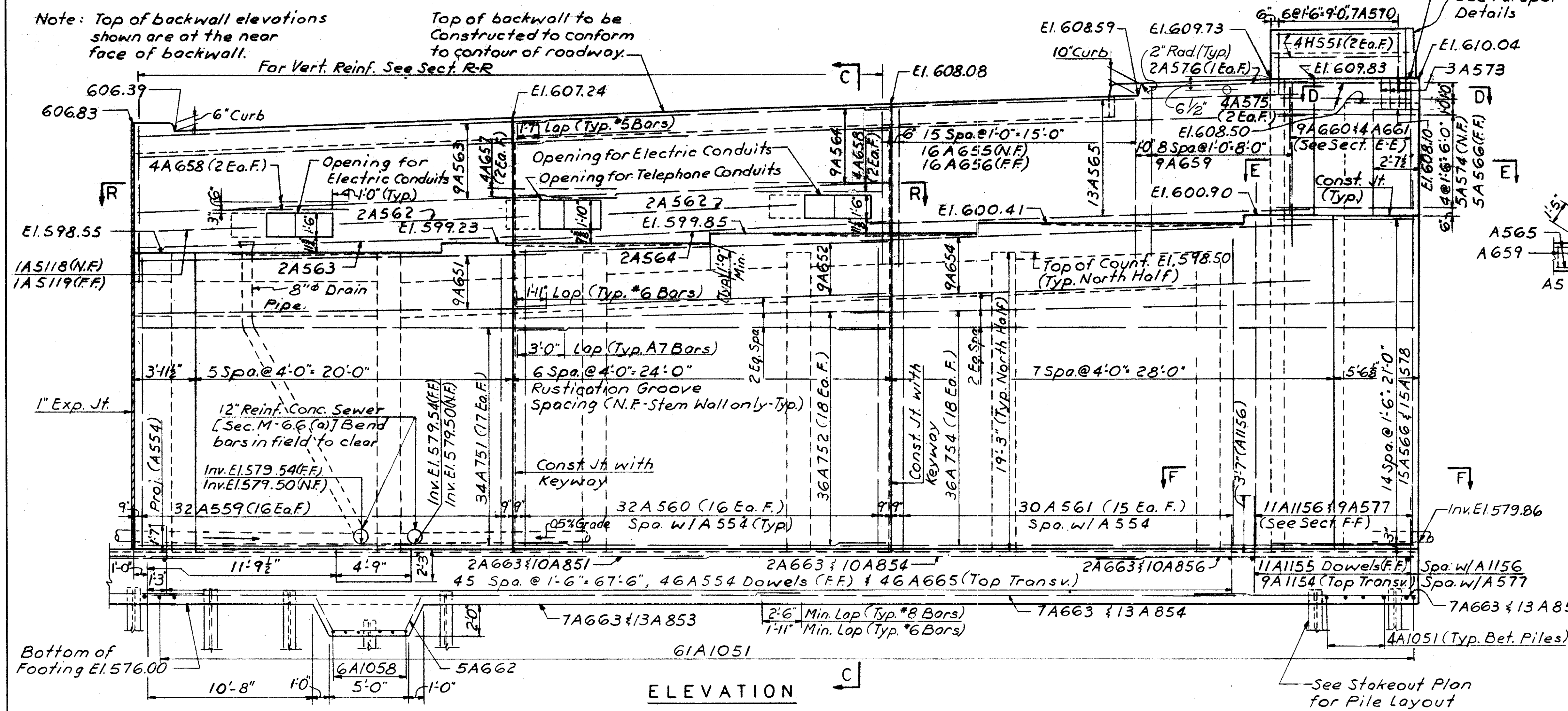
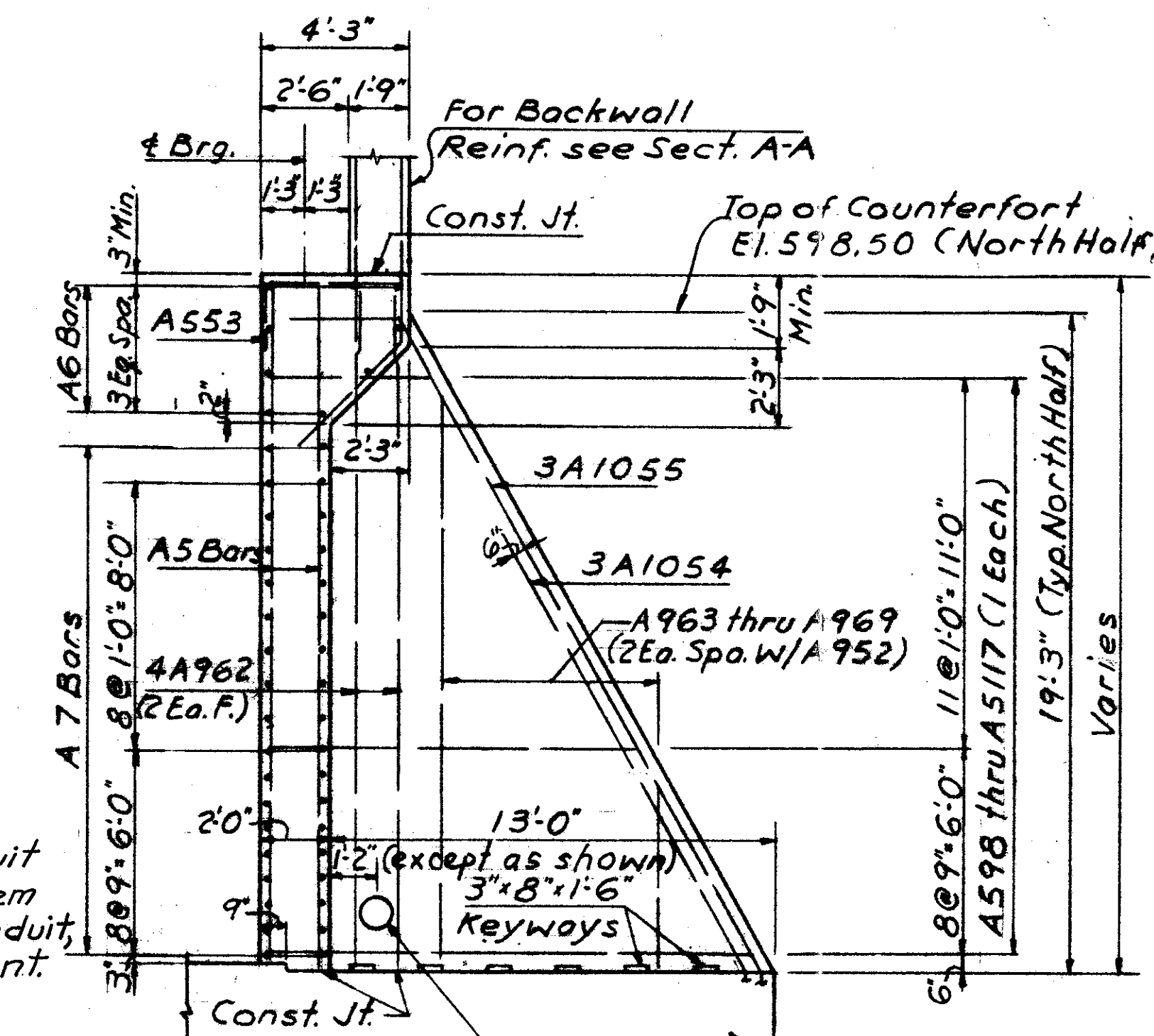
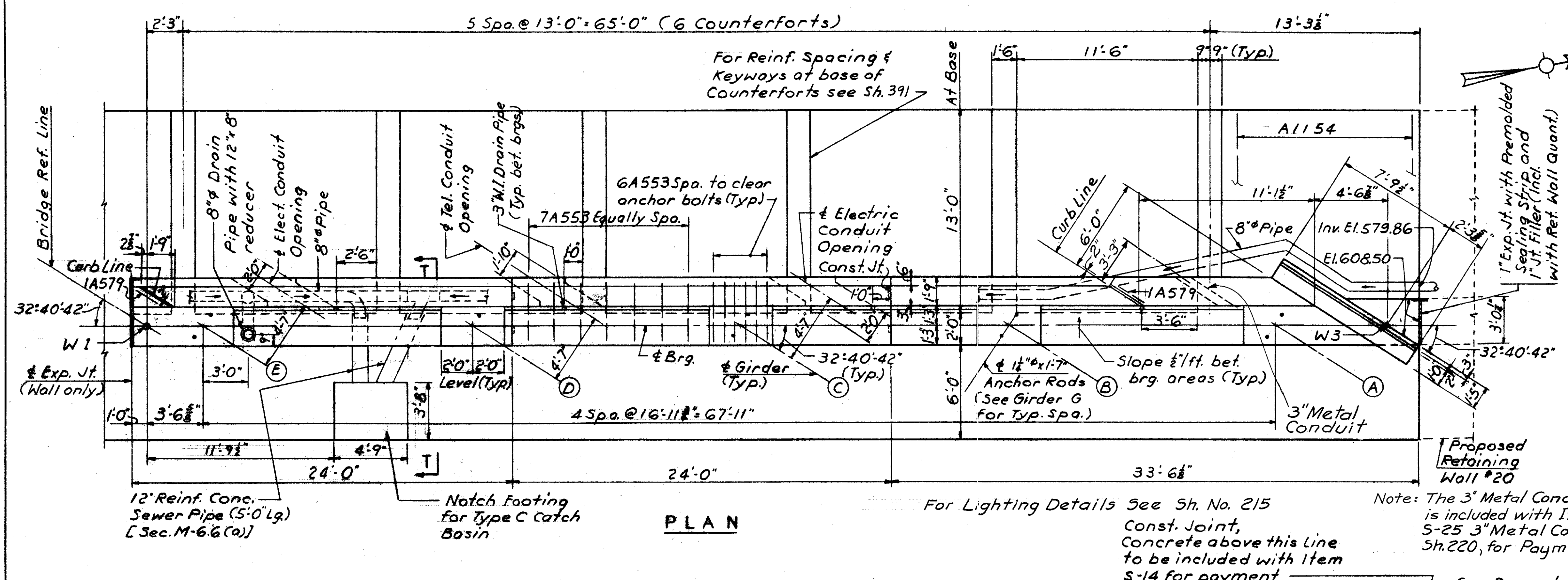
Pile Legend

⌞ Batter 1:3
 I Vertical

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FEB 20 1965

HAZLET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
STAKE OUT PLAN					
BRIDGE NO. HAM - 71-0196					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	Q.R.	GW	W.J.Z.	J.H.O. 5/20/65	

HAMILTON COUNTY
HAM-71-(1.56) (2.51)



Work this sheet with Sheet 391

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WEST ABUTMENT
BRIDGE NO. HAM-71-0196

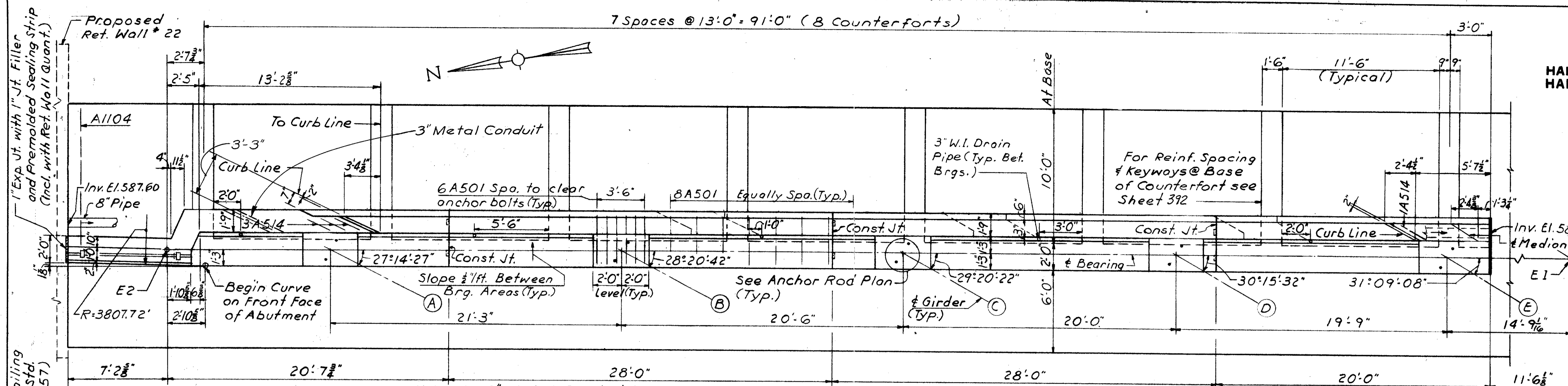
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S.U.	S.U.		W.J.Z. 3-17-65	J.H.O. 5/20/65	

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FEB 20 1965

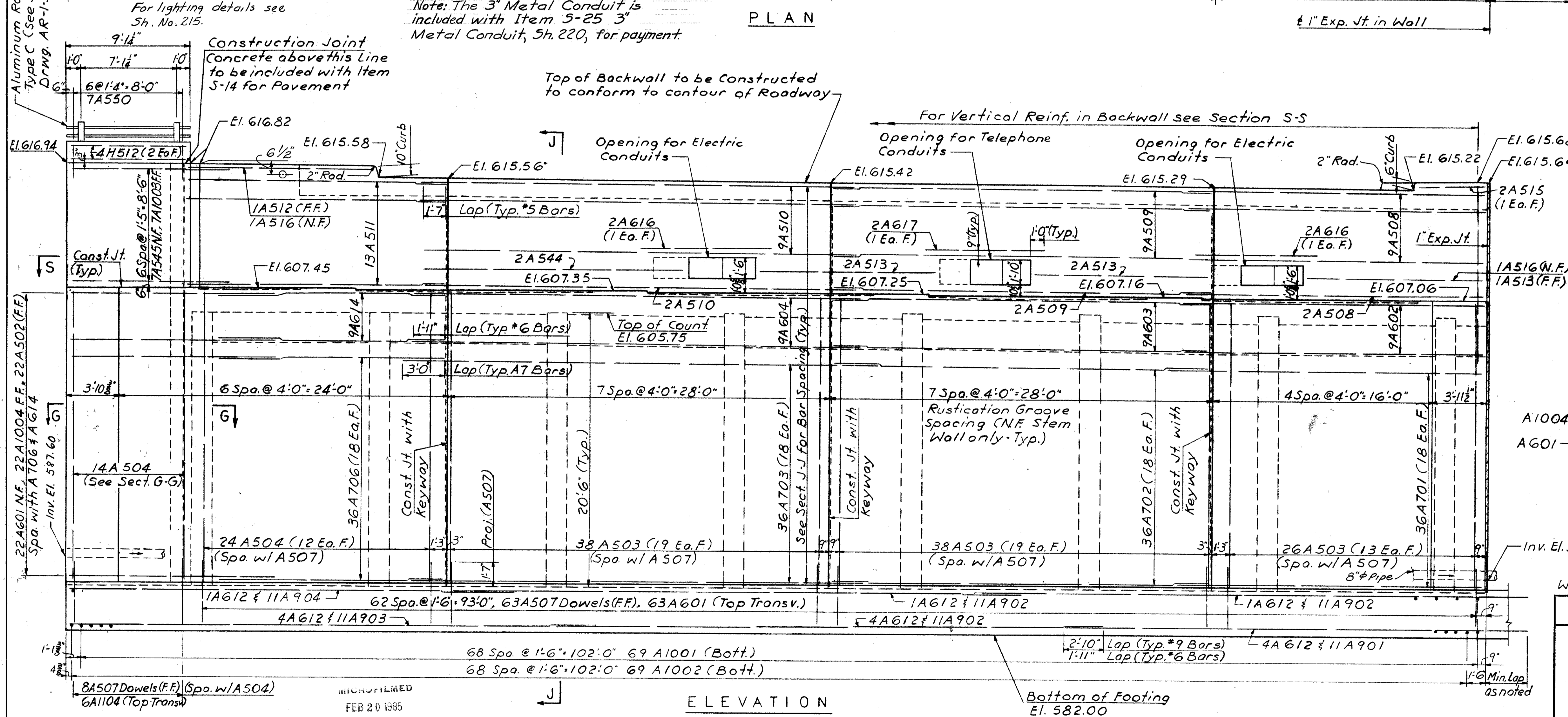
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		391 492

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

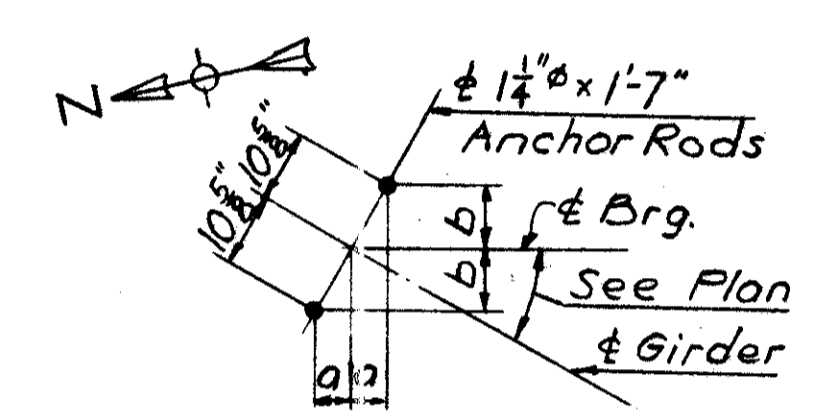
ANCHOR ROD LAYOUT DIM.		
Girder	a	b
A	4 3/8"	9 7/8"
B	5 7/8"	9 3/4"
C	5 3/8"	9 1/4"
D	5 3/8"	9 7/8"
E	5 1/2"	9 3/4"
F	5 1/8"	9"
G	5 1/8"	9"
H	5 3/8"	8 15/16"
J	5 3/8"	8 3/8"
K	5 3/8"	8 3/8"



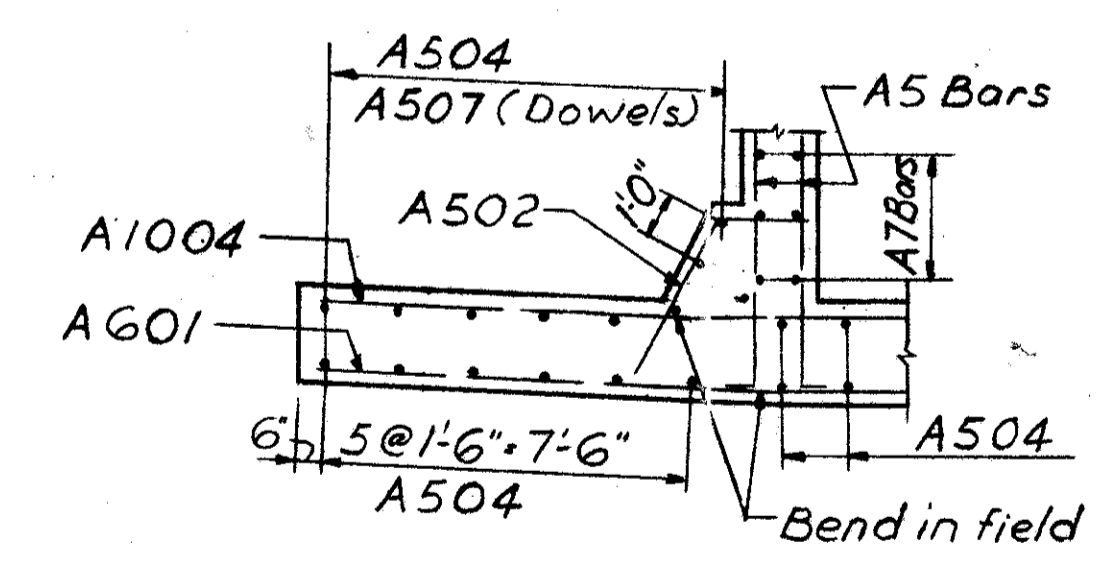
PLAN



ELEVATION



ANCHOR ROD PLAN
(See Table for Dim. of a & b)



SECTION G-G

Work this sheet with Sheets 392 & 393
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CINCINNATI, OHIO

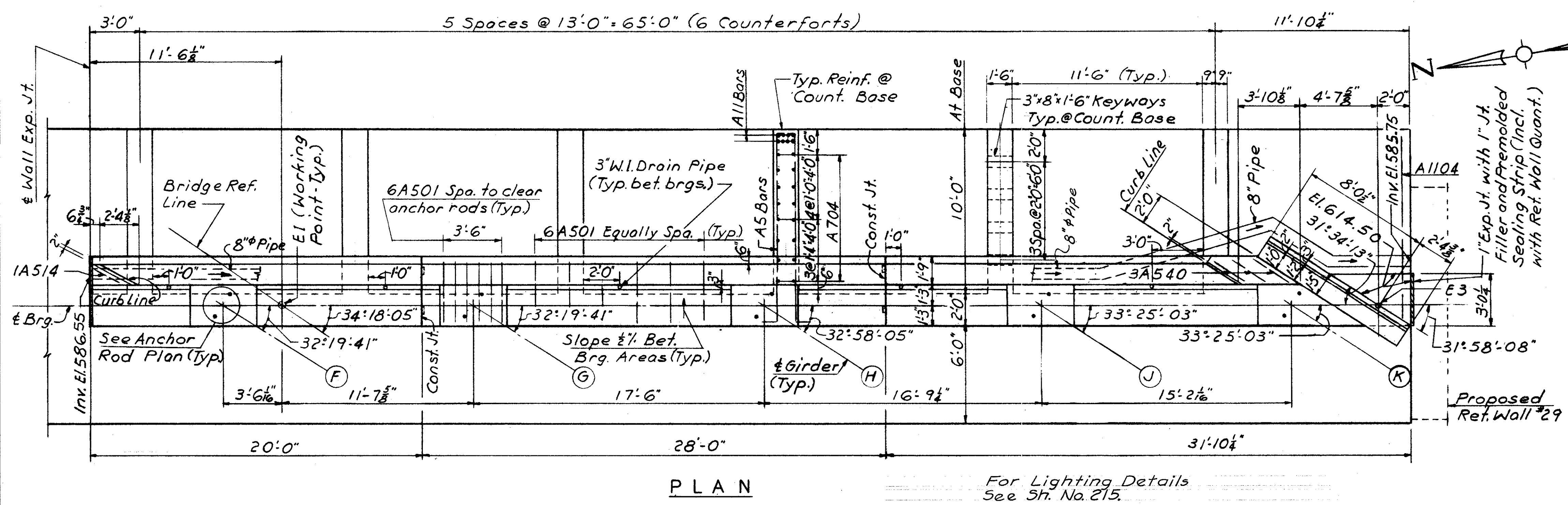
**EAST ABUTMENT
BRIDGE NO. HAM-71-0196**

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S.U.	S.U.		M.L.Z.	J.H.O. 5/20/65	

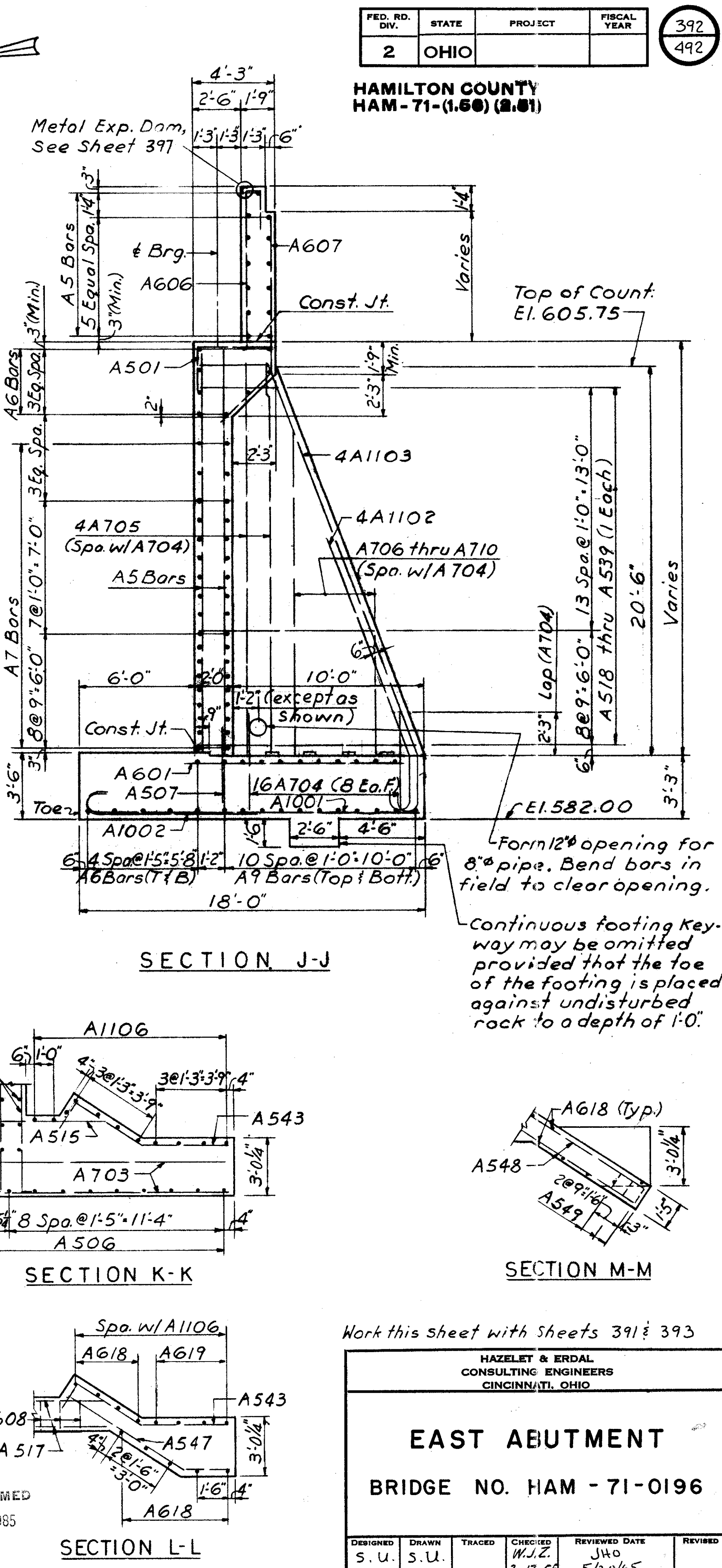
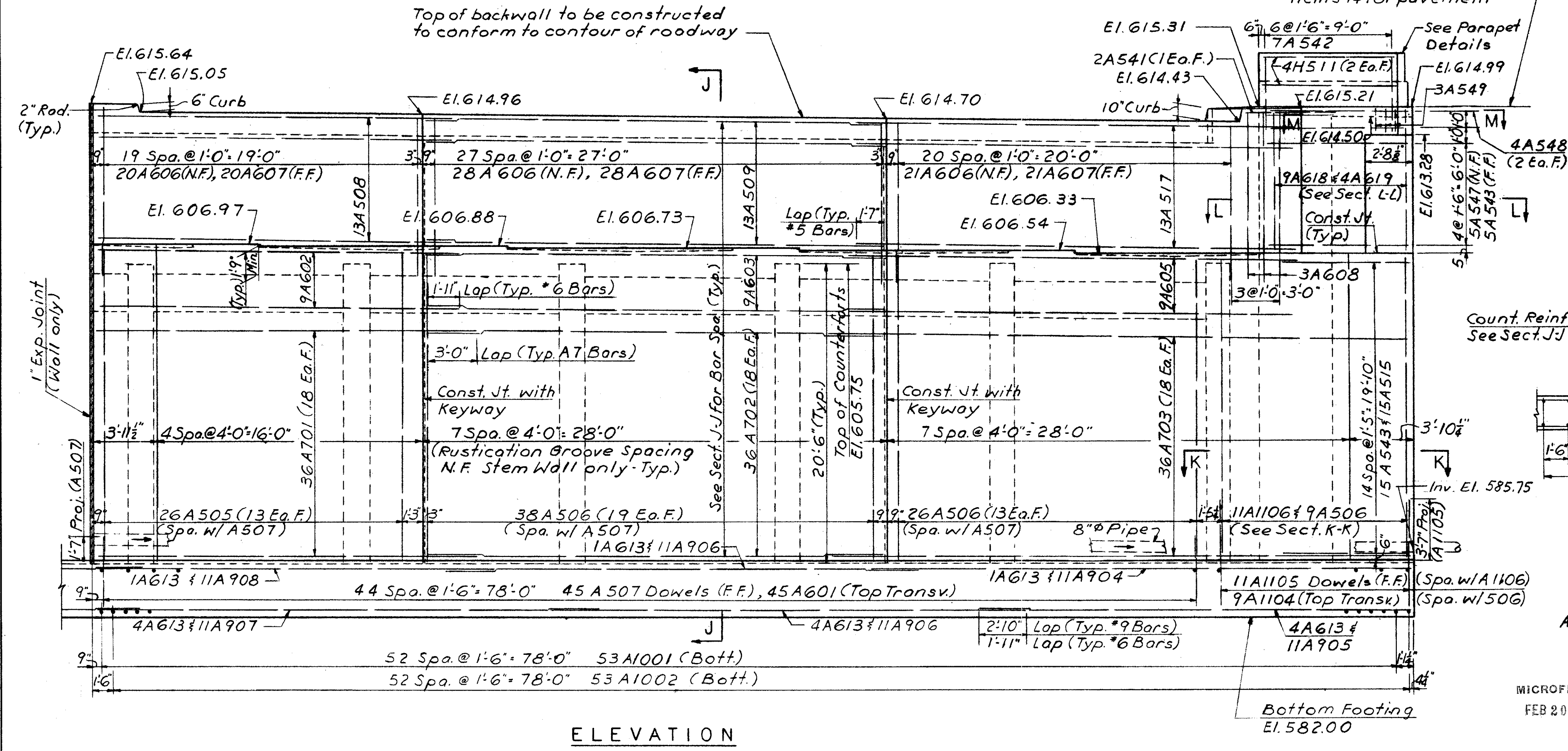
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FEB 20 1985

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR	392 492
2	OHIO			

HAMILTON COUNTY
HAM - 71 - (1.60) (2.81)



Note: Top of backwall elevations shown are at near face of backwall.



Work this sheet with Sheets 391 & 393

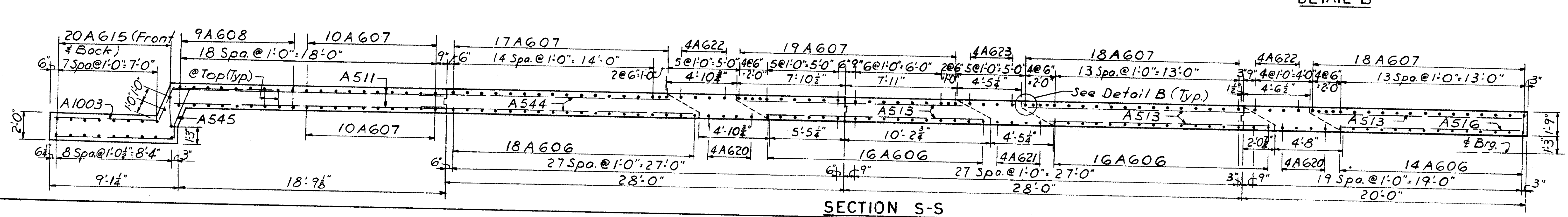
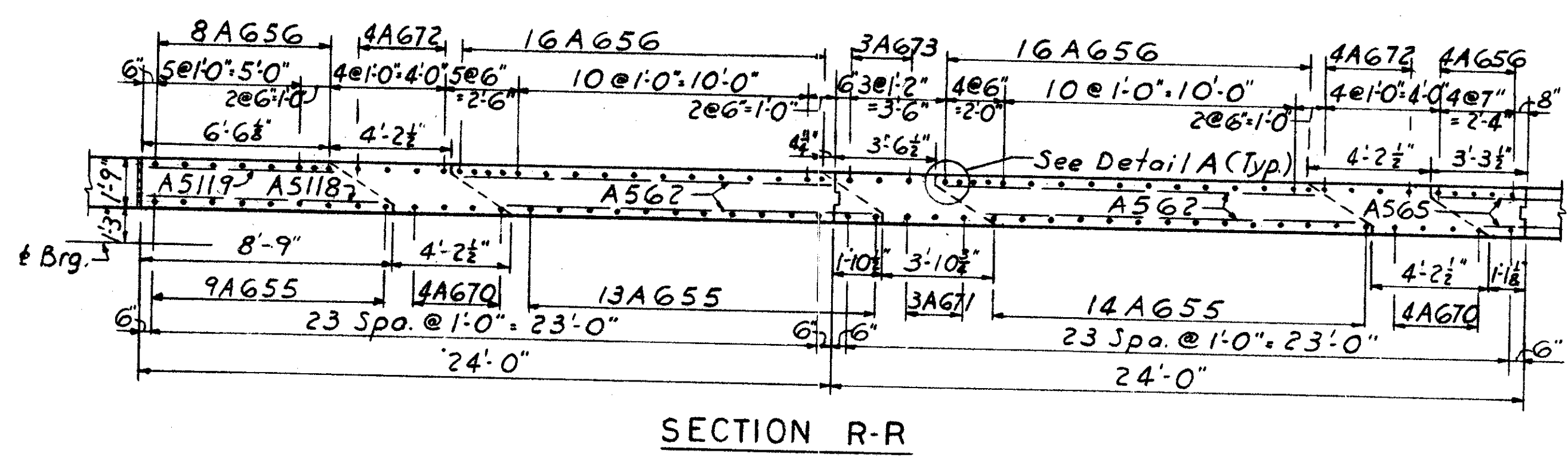
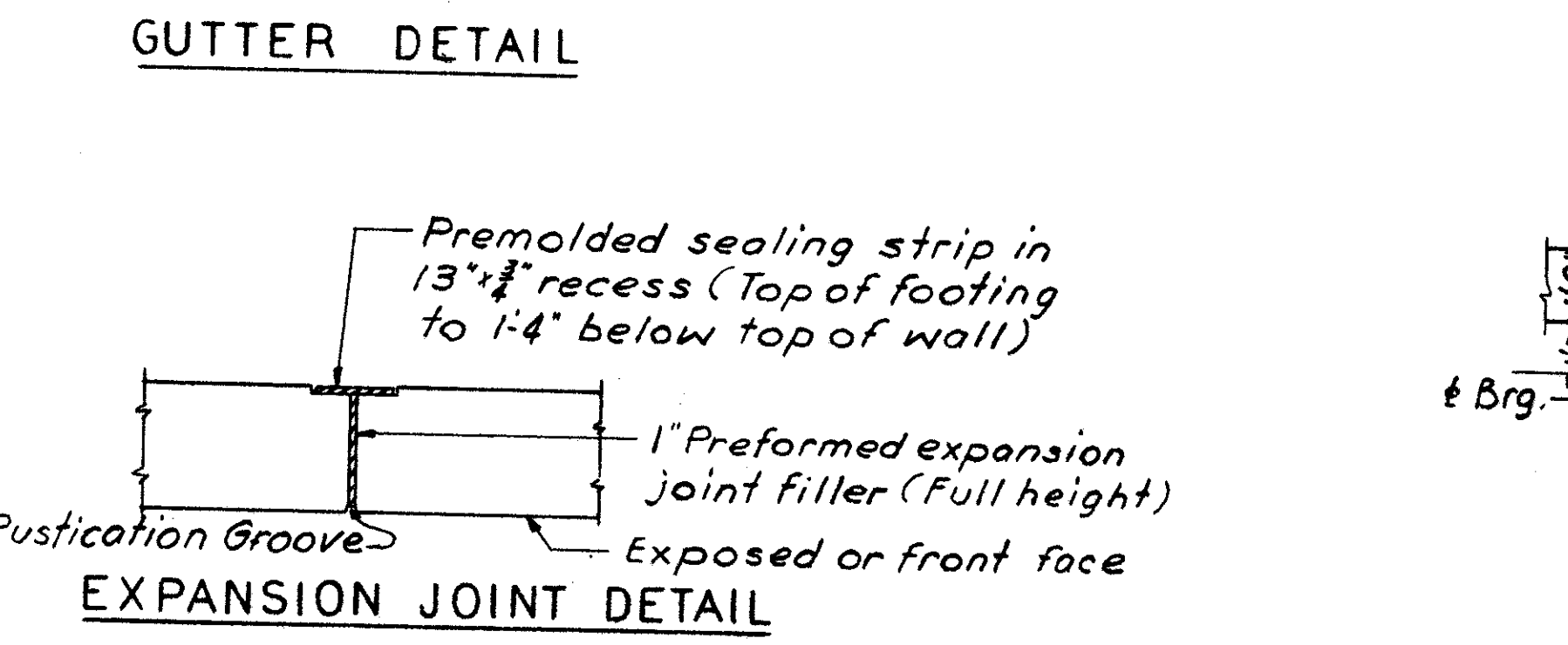
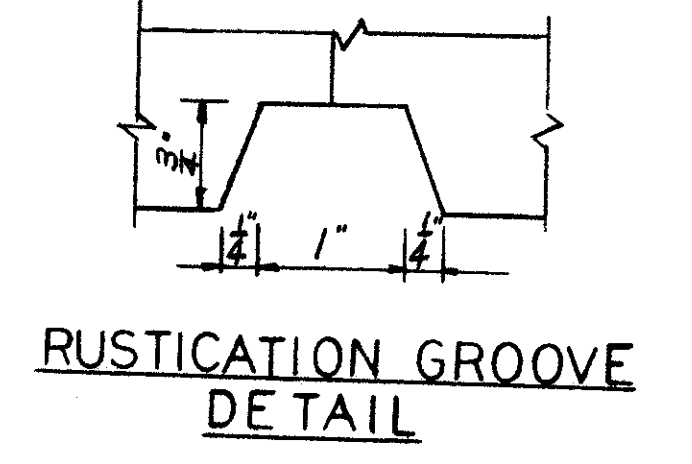
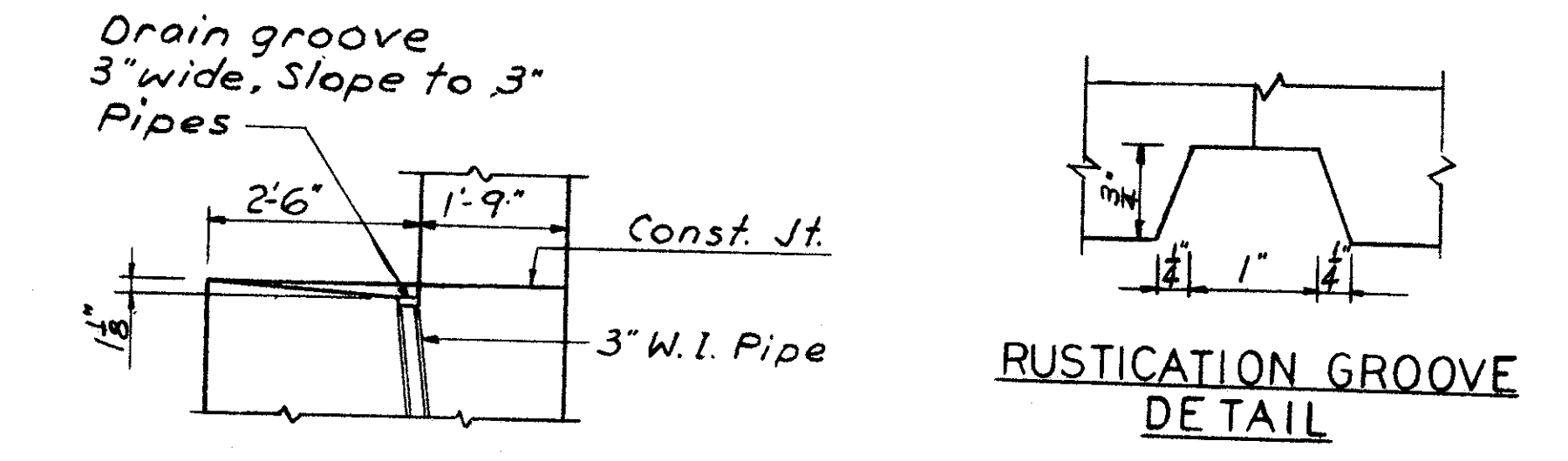
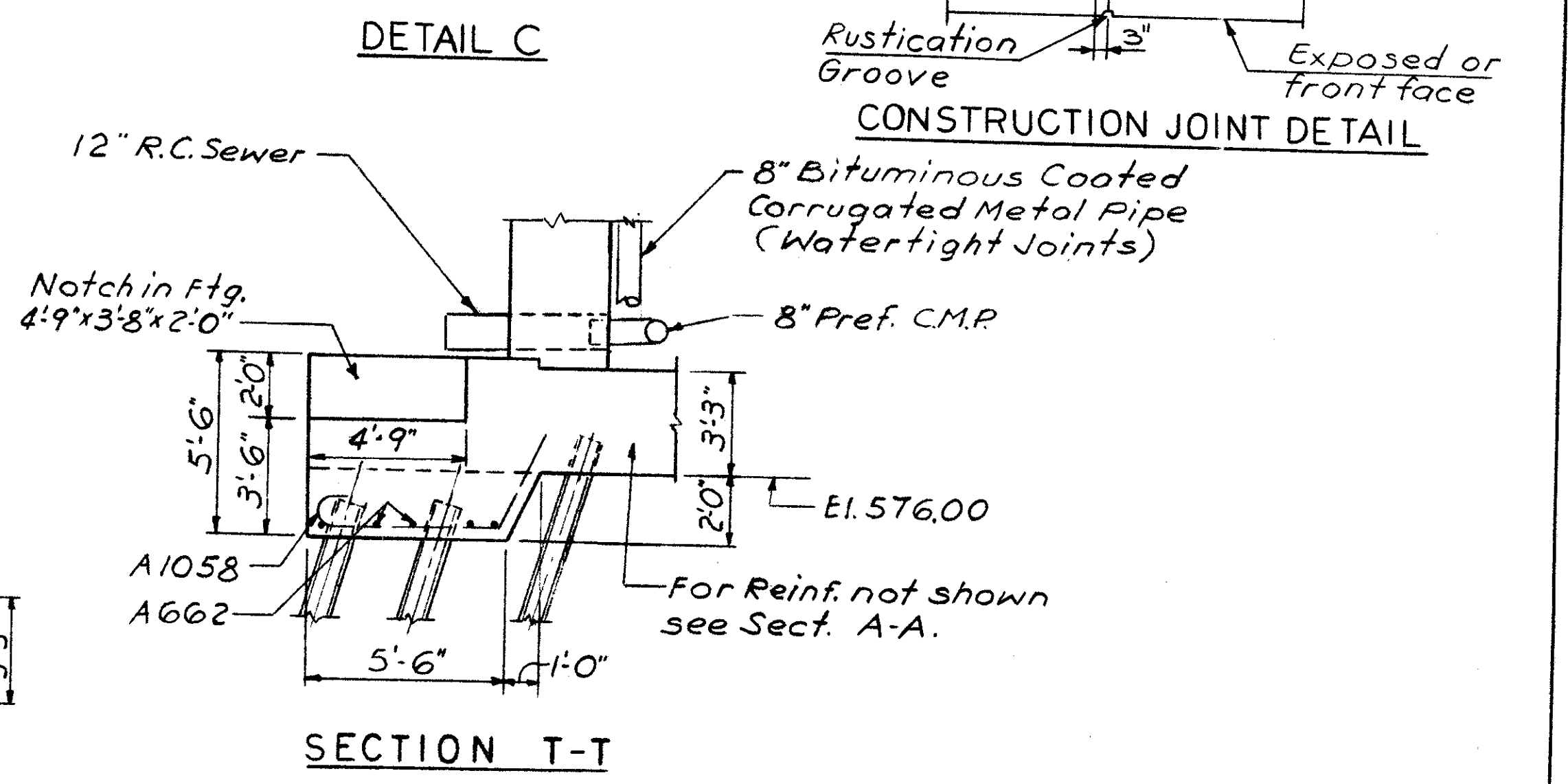
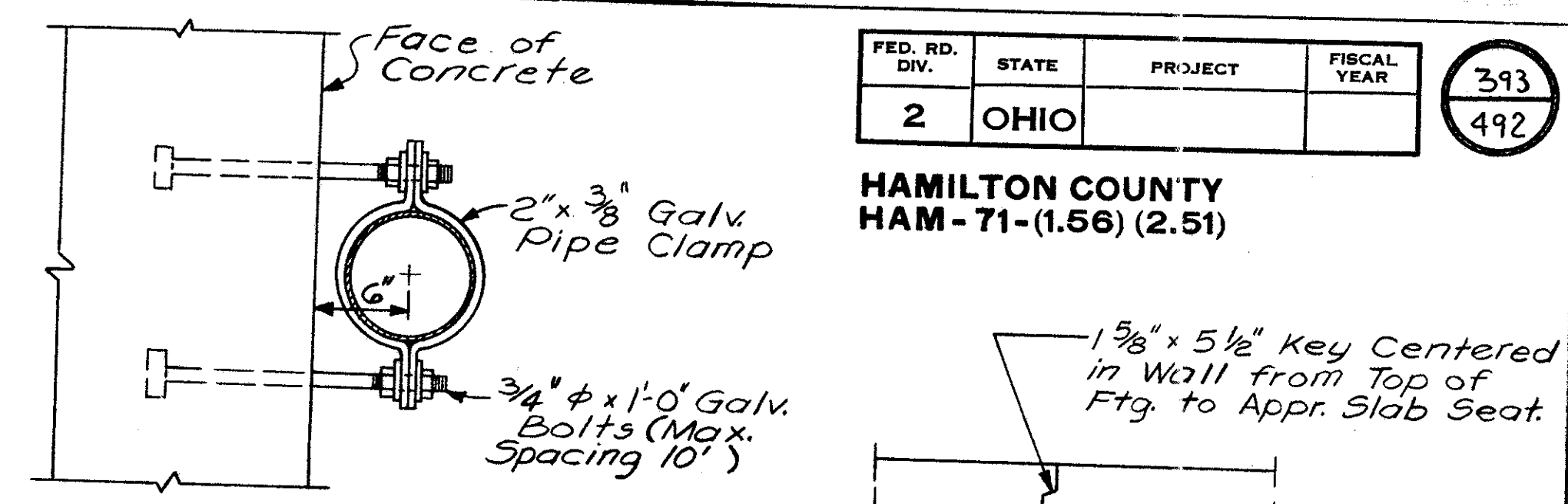
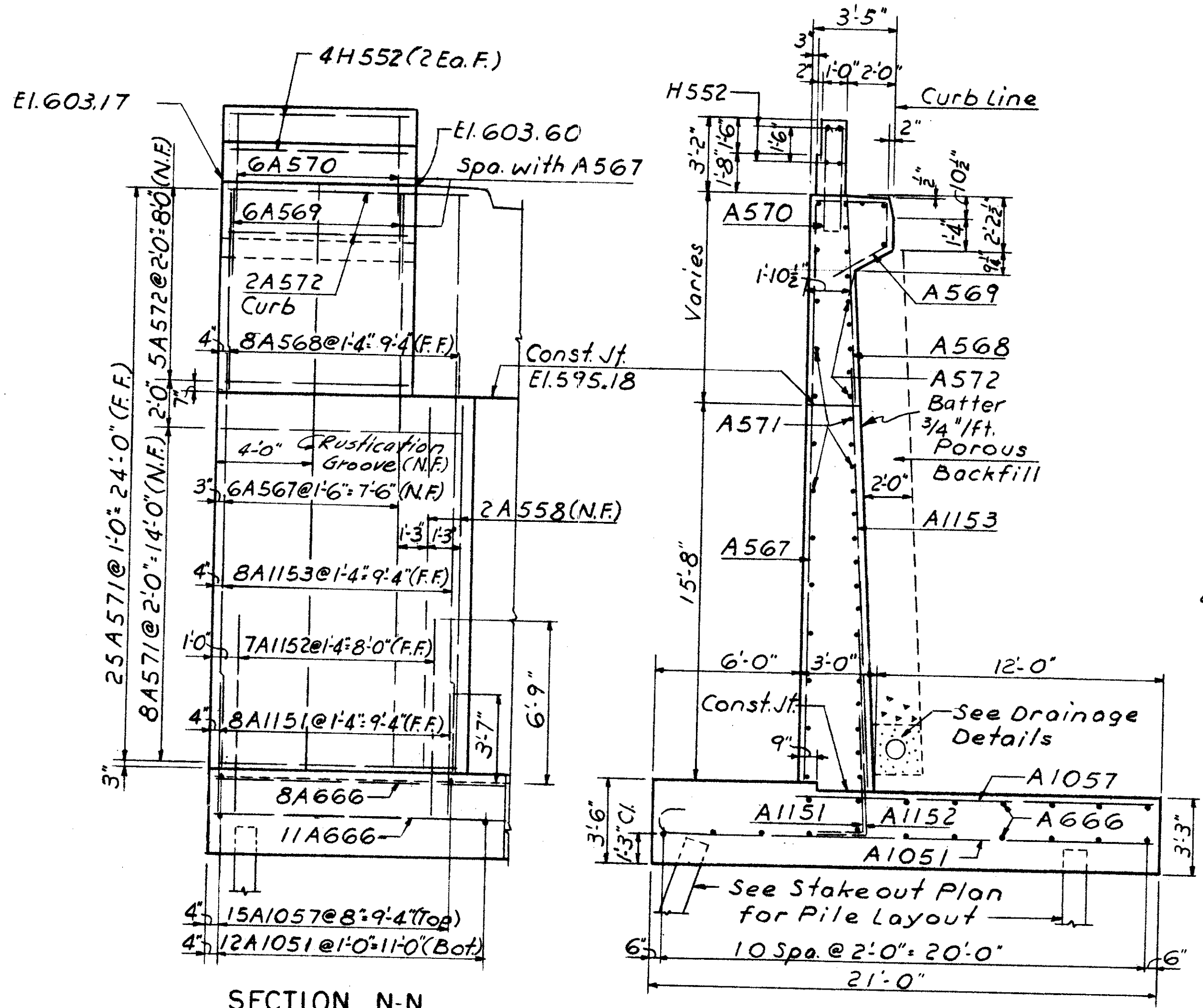
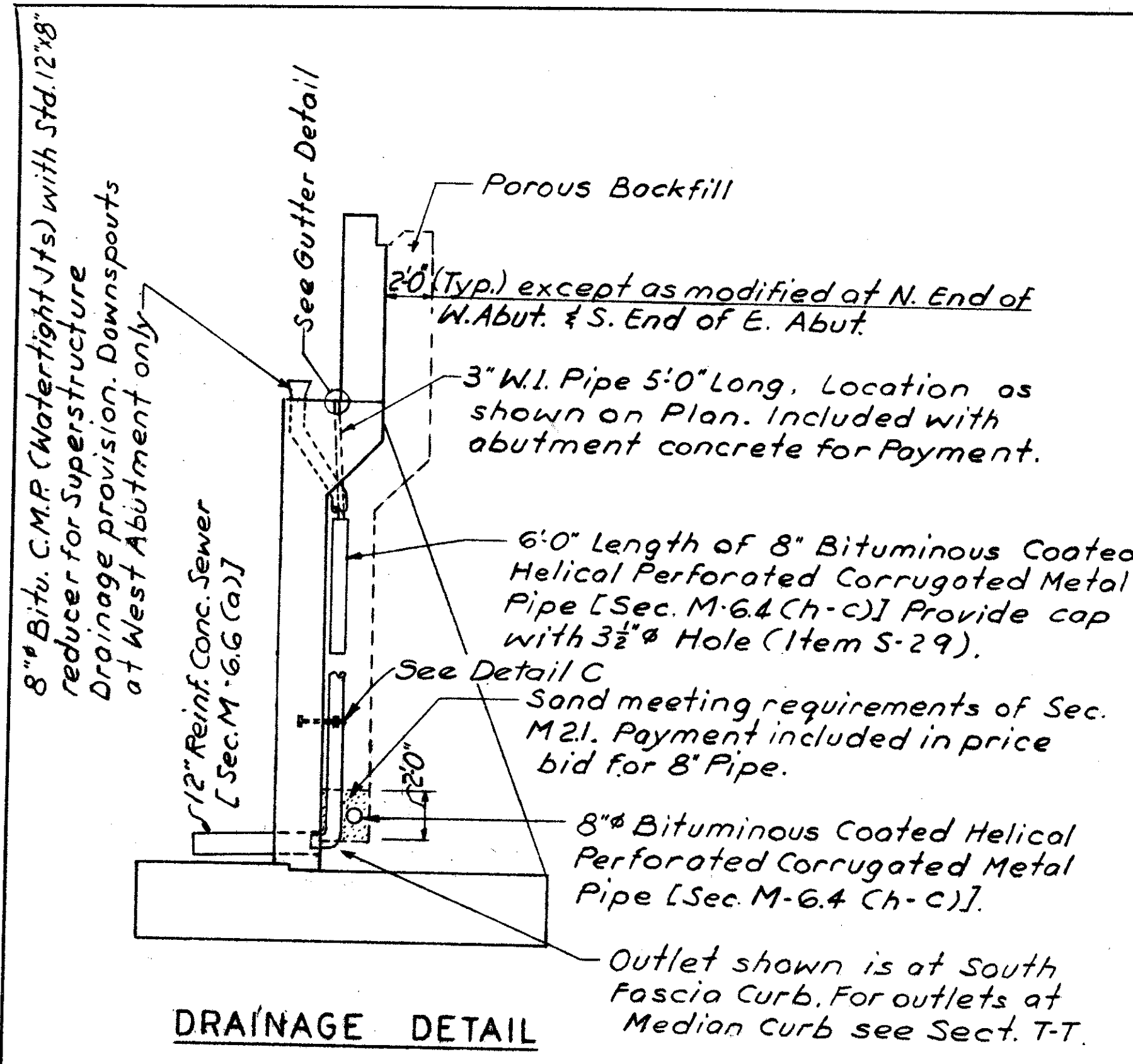
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CONSULTING ENGINEERS
CINCINNATI, OHIO

EAST ABUTMENT
BRIDGE NO. HAM - 71 - 0196

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
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			3-17-65	5/12/65	

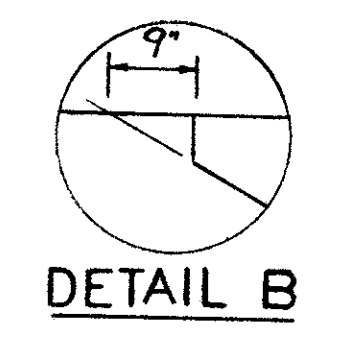
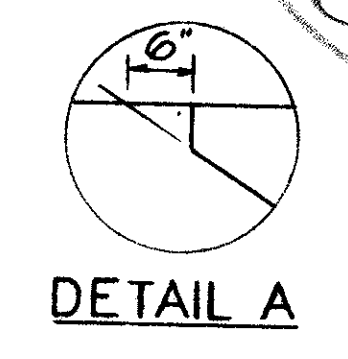
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HAMILTON COUNTY
HAM-71-(1.56) (2.51)



Procedure:
Before the backwall is constructed, the embankment shall be placed and compacted up to the level of the subgrade with a 1:1 slope from the bridge seat to the subgrade, for a distance of 200 ft. back of the West Abutment only. Porous Backfill 2 ft. thick, full length of abutment and wings shall extend up to the underside of the approach slab or to the finished ground surface.

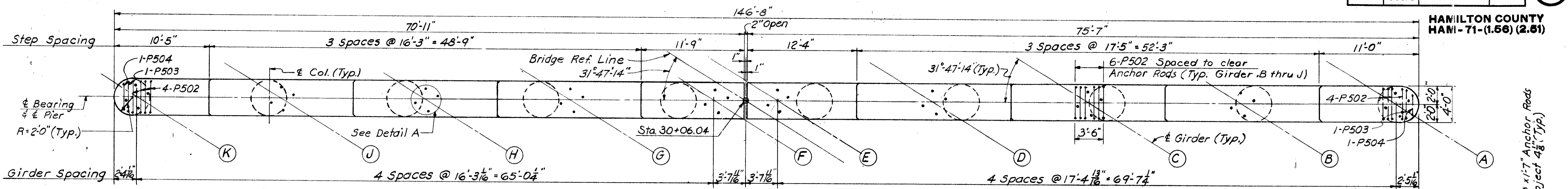
Notes:
Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes. Provide 3" clearance to reinforcing steel in footing and 2" in wall (Min.) N.F. denotes near face, F.F. denotes far face. Ea. F. denotes each face. Abutment concrete shall be Class "E". Concrete above bridge seat construction joint shall not be placed until superstructure steel is in place.



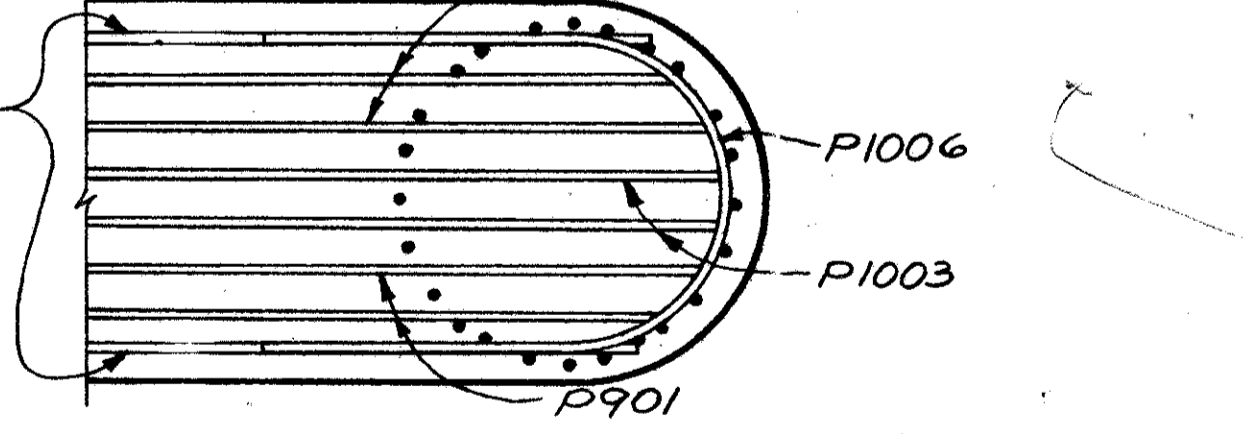
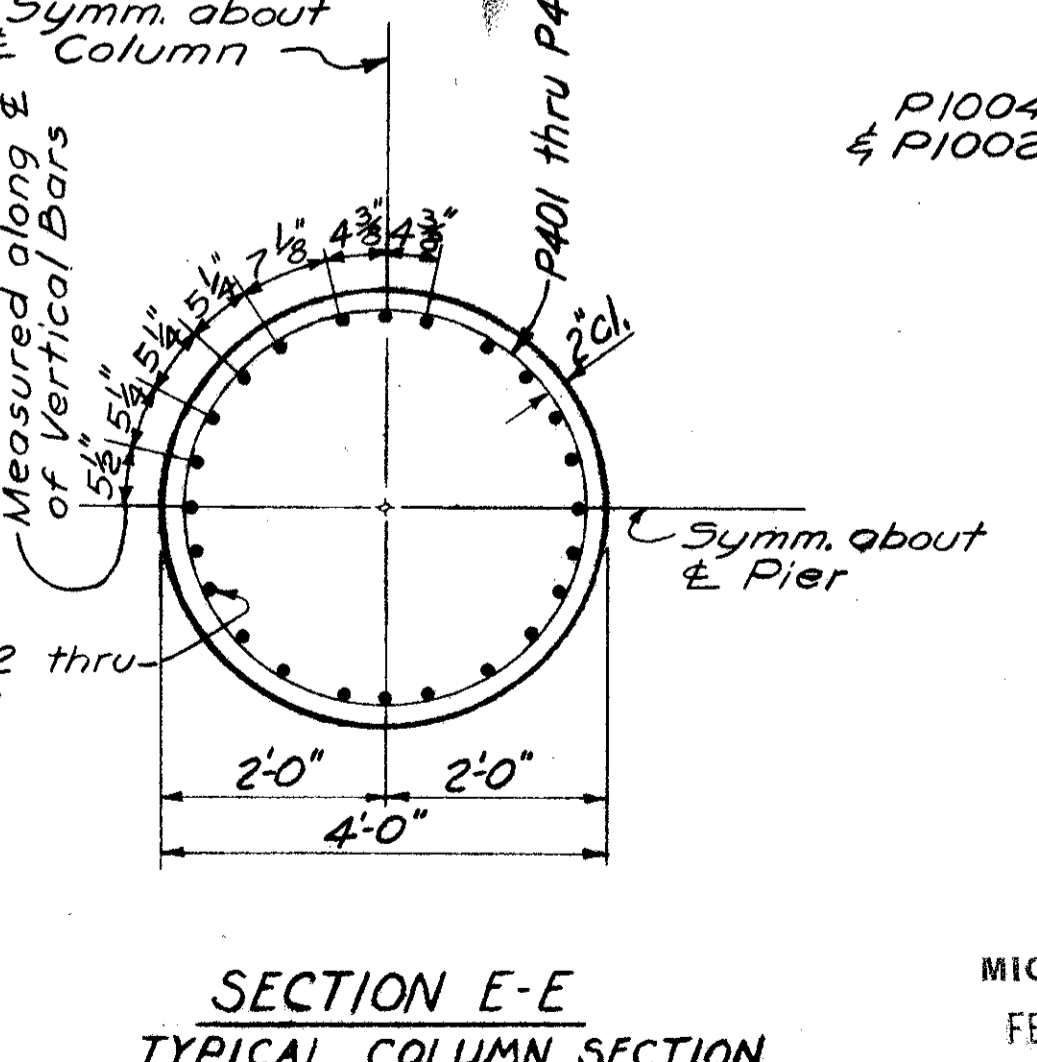
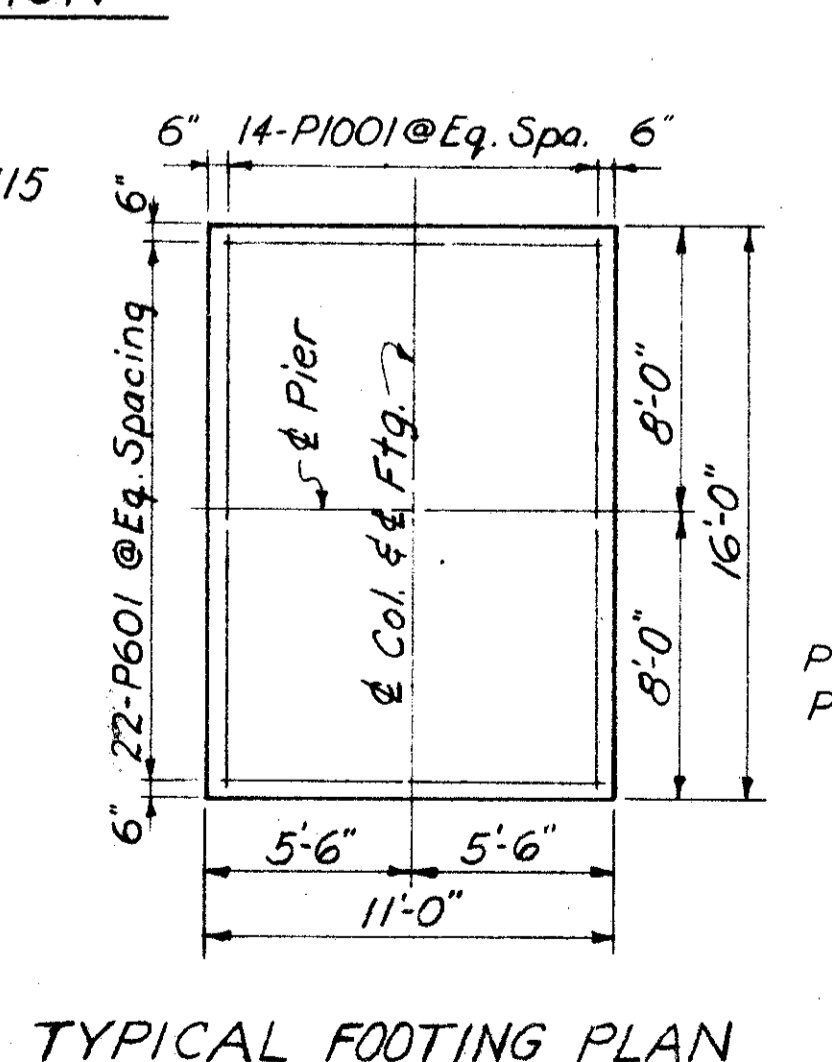
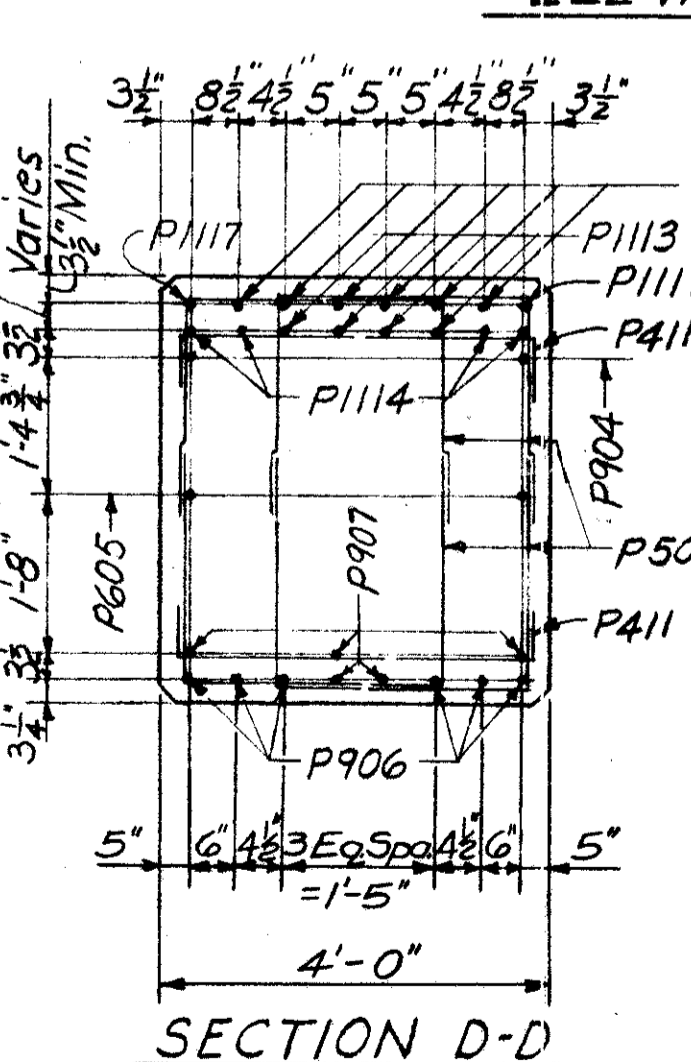
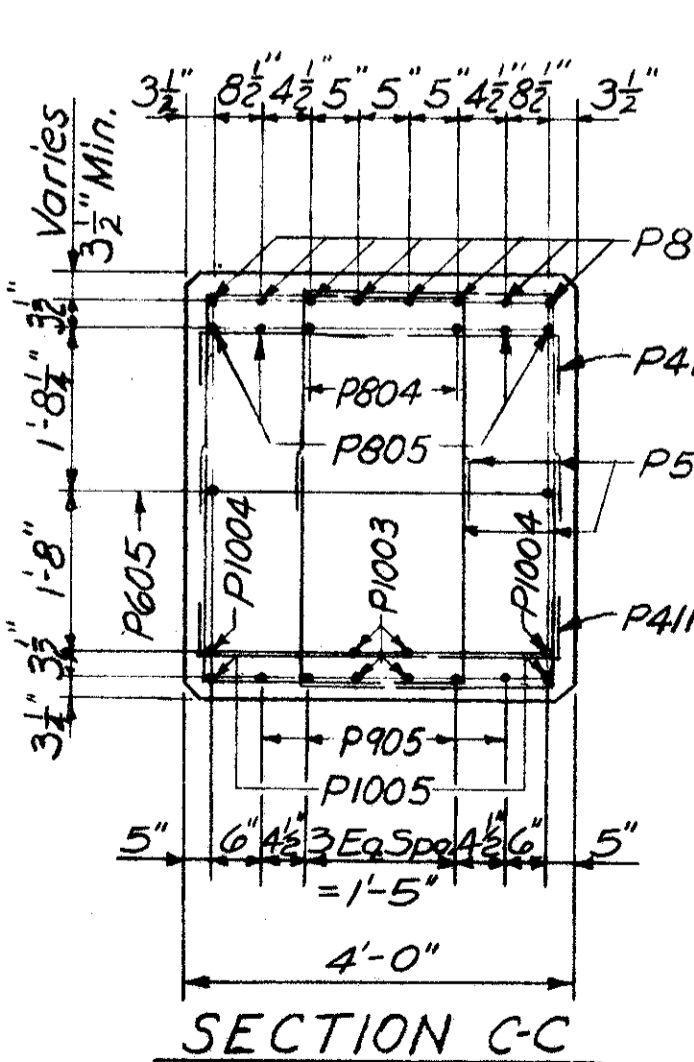
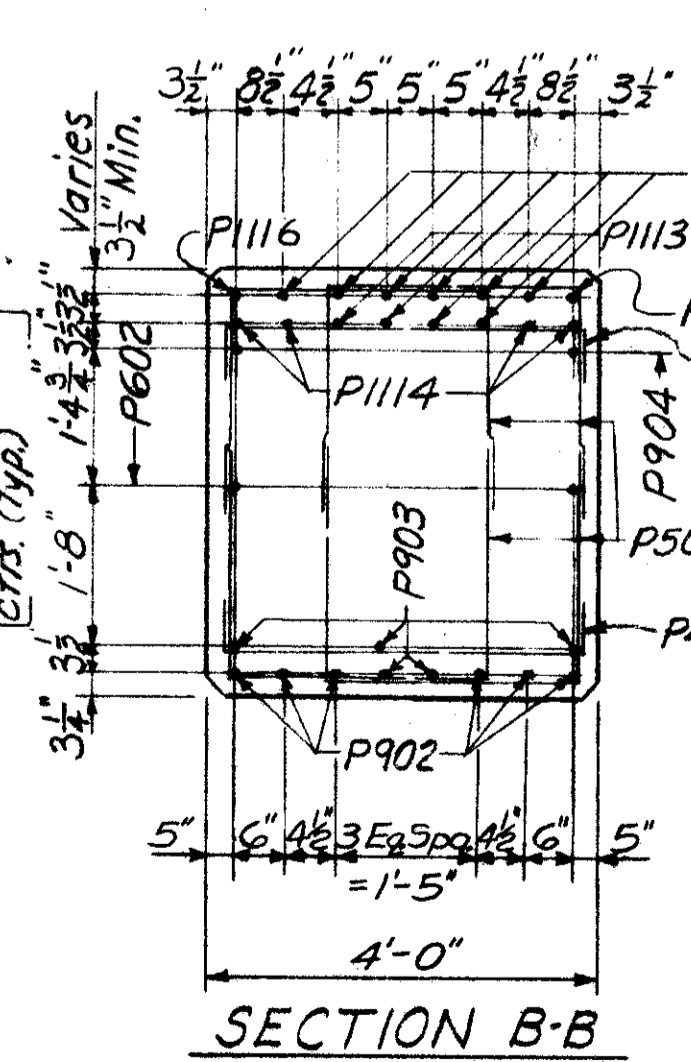
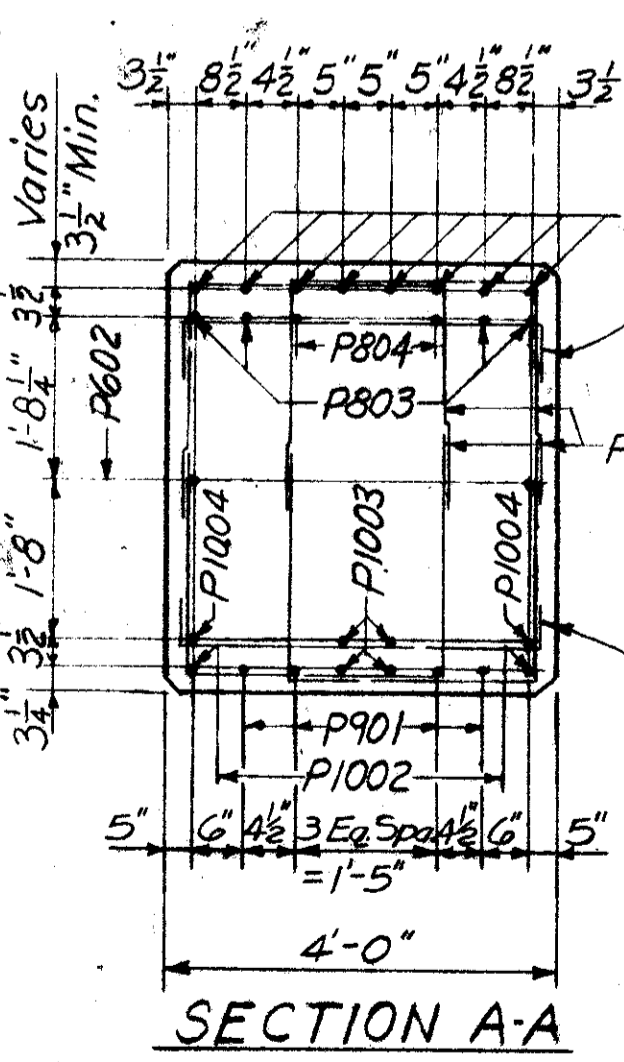
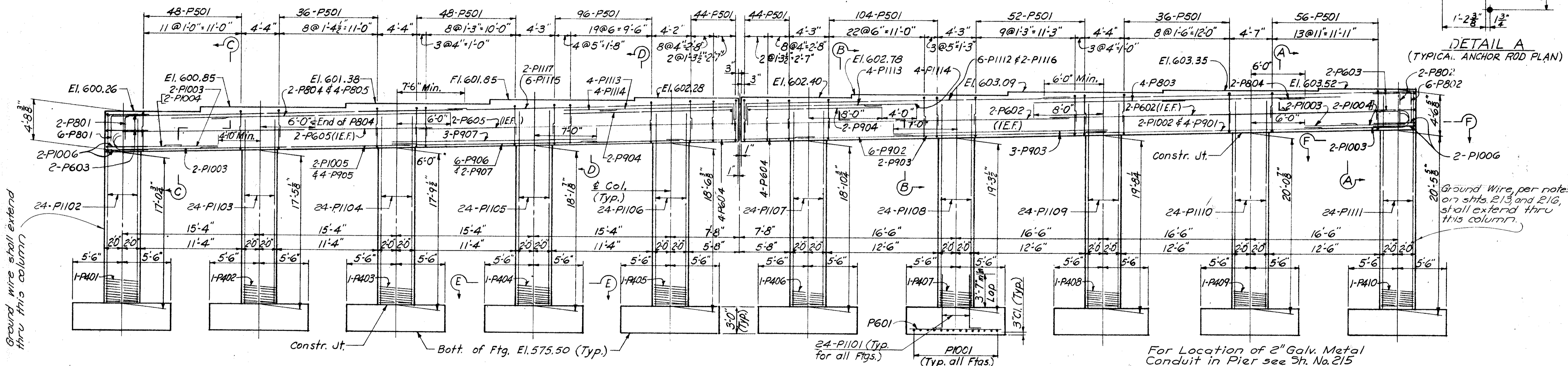
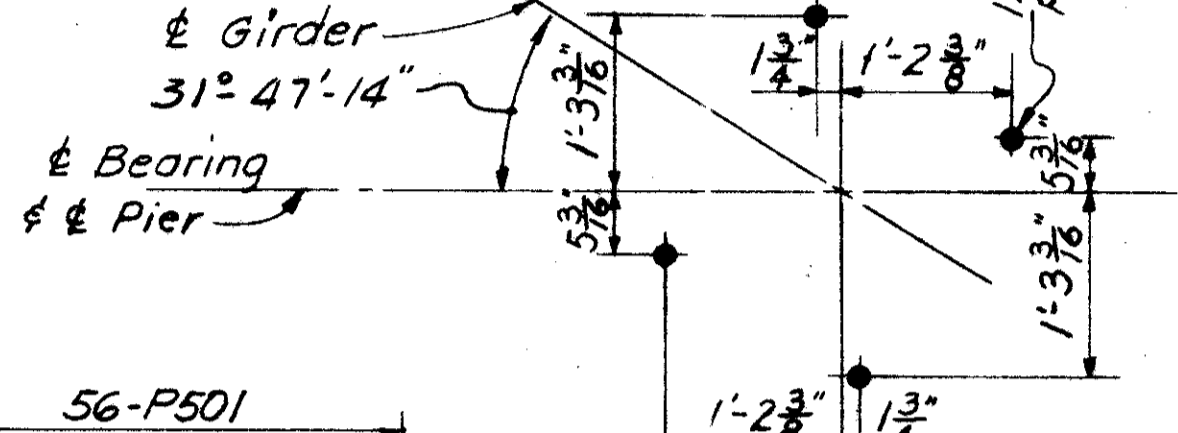
Work this sheet with Sheets 391 & 392

HAZLET & IRDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

ABUTMENT DETAILS
BRIDGE NO. HAM-71-0196



CAP PLAN



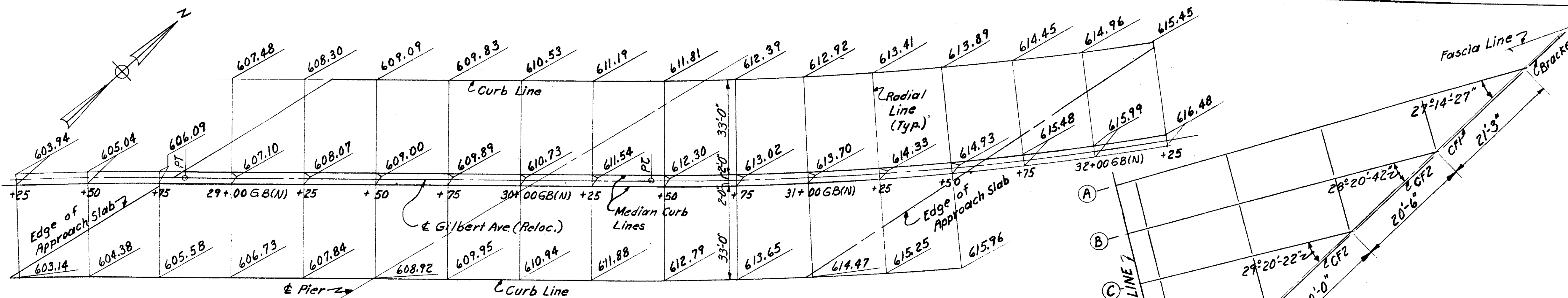
HAZELT & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

PIER DETAILS
BRIDGE NO. HAM-71-0196

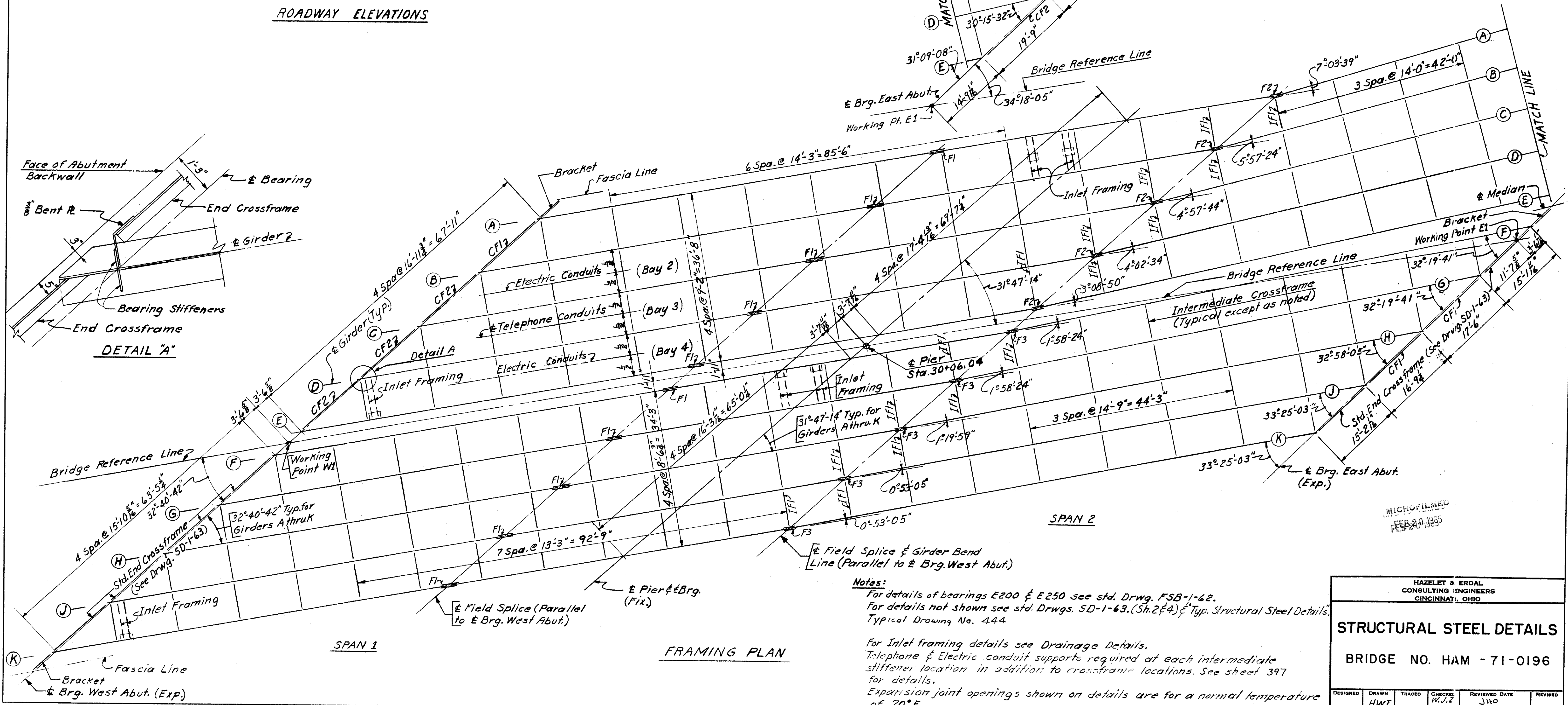
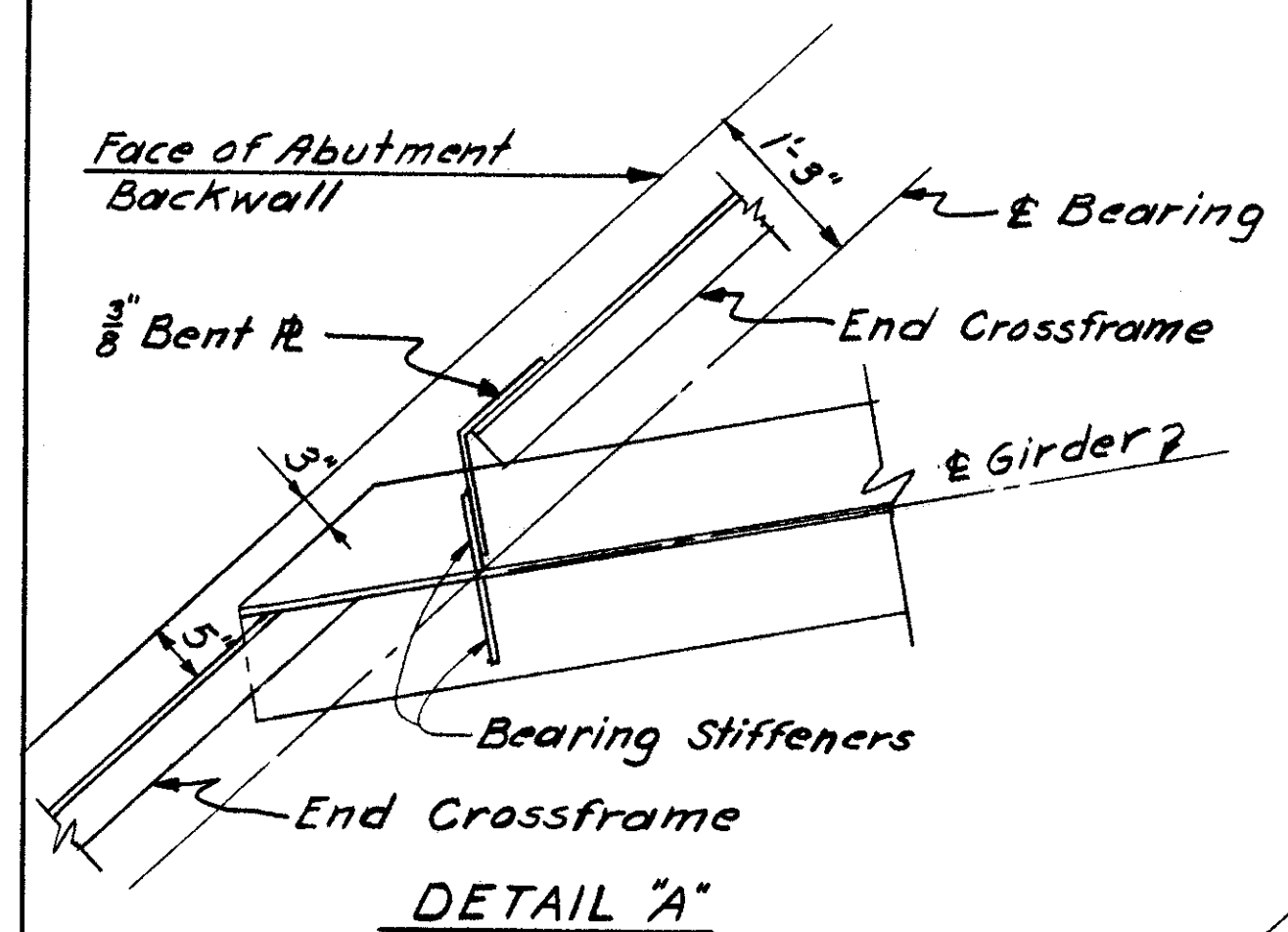
MICROFILMED
FEB 20 1965

DESIGNED D.W.	DRAWN C.B.H.	TRACED	CHECKED D.V. 3-17-65	REVIEWED DATE J.V.G. 5/20/65	REVISIONS
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HAMILTON COUNTY
HAM-71-(1.56) (2.61)



ROADWAY ELEVATIONS



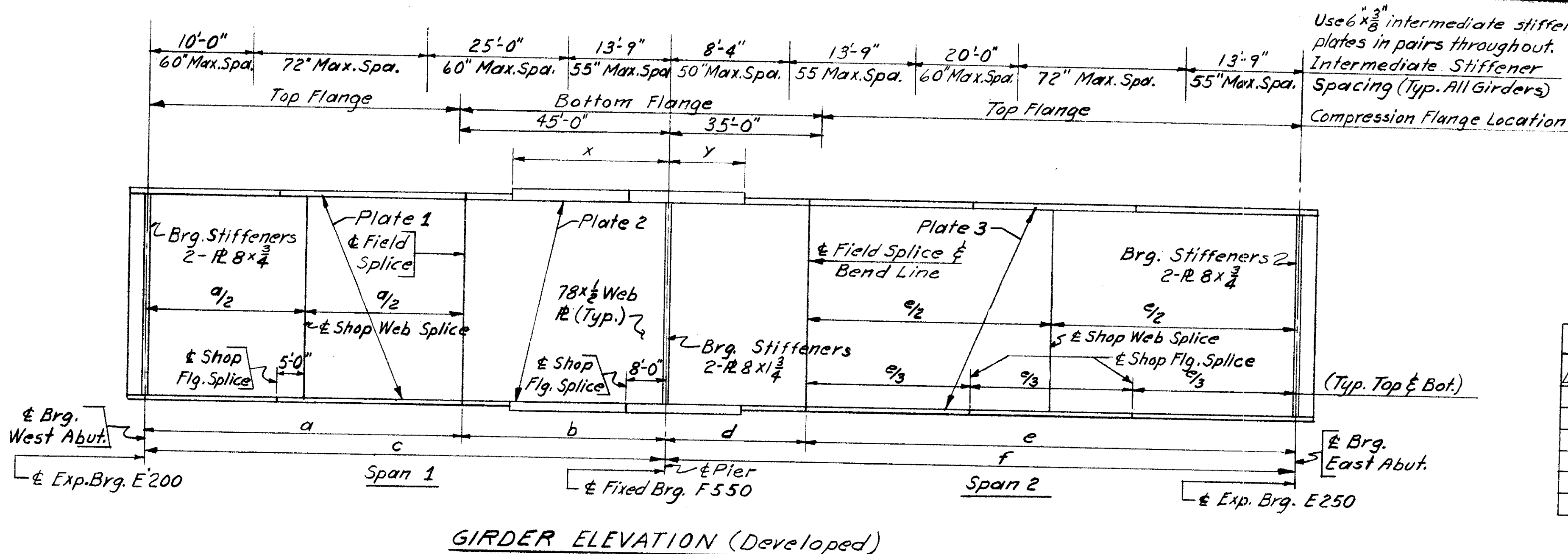
FRAMING PLAN

Notes:
 For details of bearings E200 & E250 see std. Drwg. FSB-1-62.
 For details not shown see std. Drwgs. SD-1-63. (Sh. 2 & 4) & Typ. Structural Steel Details, Typical Drawing No. 444.
 For Inlet framing details see Drainage Details.
 Telephone & Electric conduit supports required at each intermediate stiffener location in addition to crossframe locations. See sheet 397 for details.
 Expansion joint openings shown on details are for a normal temperature of 70°F.

MICROFILMED
FEB 20 1985

HAZLET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
STRUCTURAL STEEL DETAILS					
BRIDGE NO. HAM - 71-0196					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVIEWED
	H.W.T.		W.J.E.	3-17-65	J.H.O.
				5/20/65	

HAMILTON COUNTY HAM-71-(1.56) (2.51)

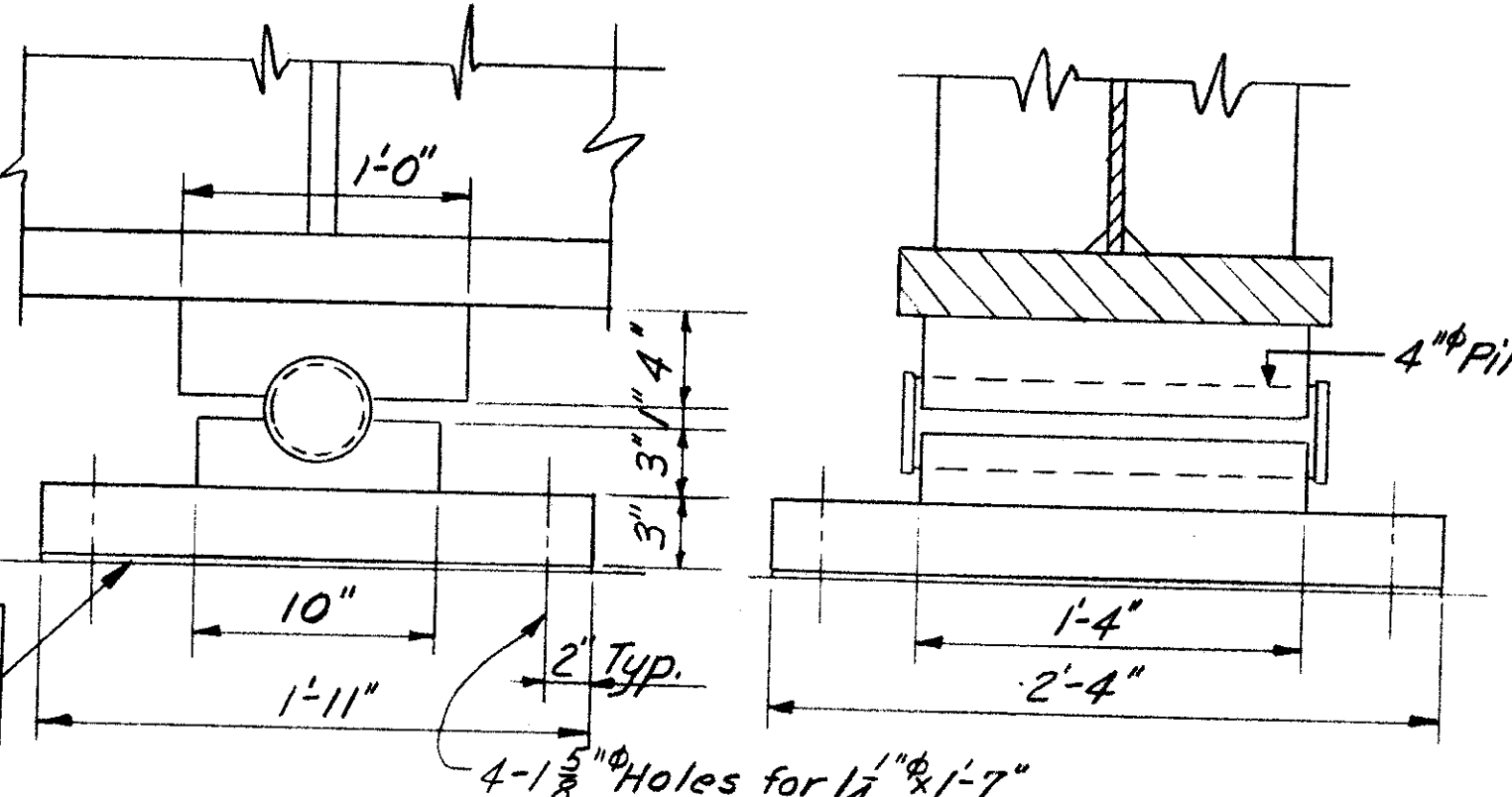
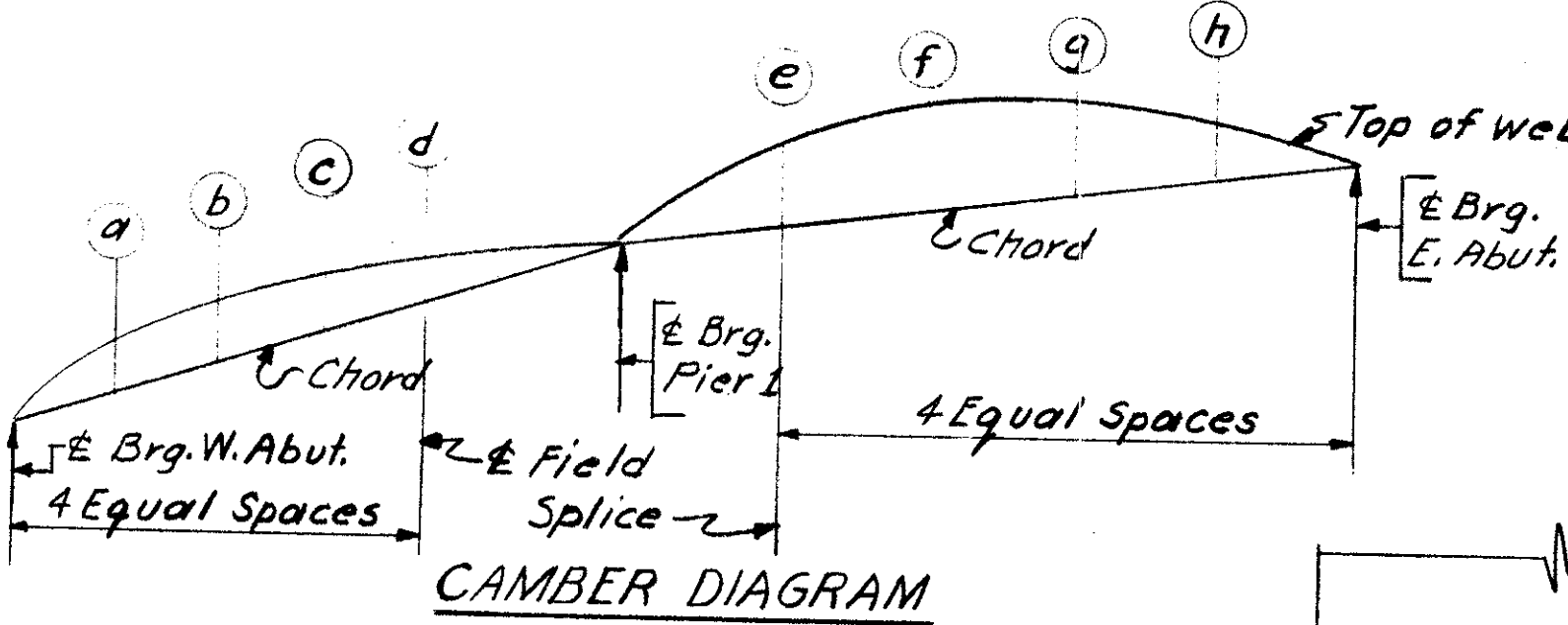
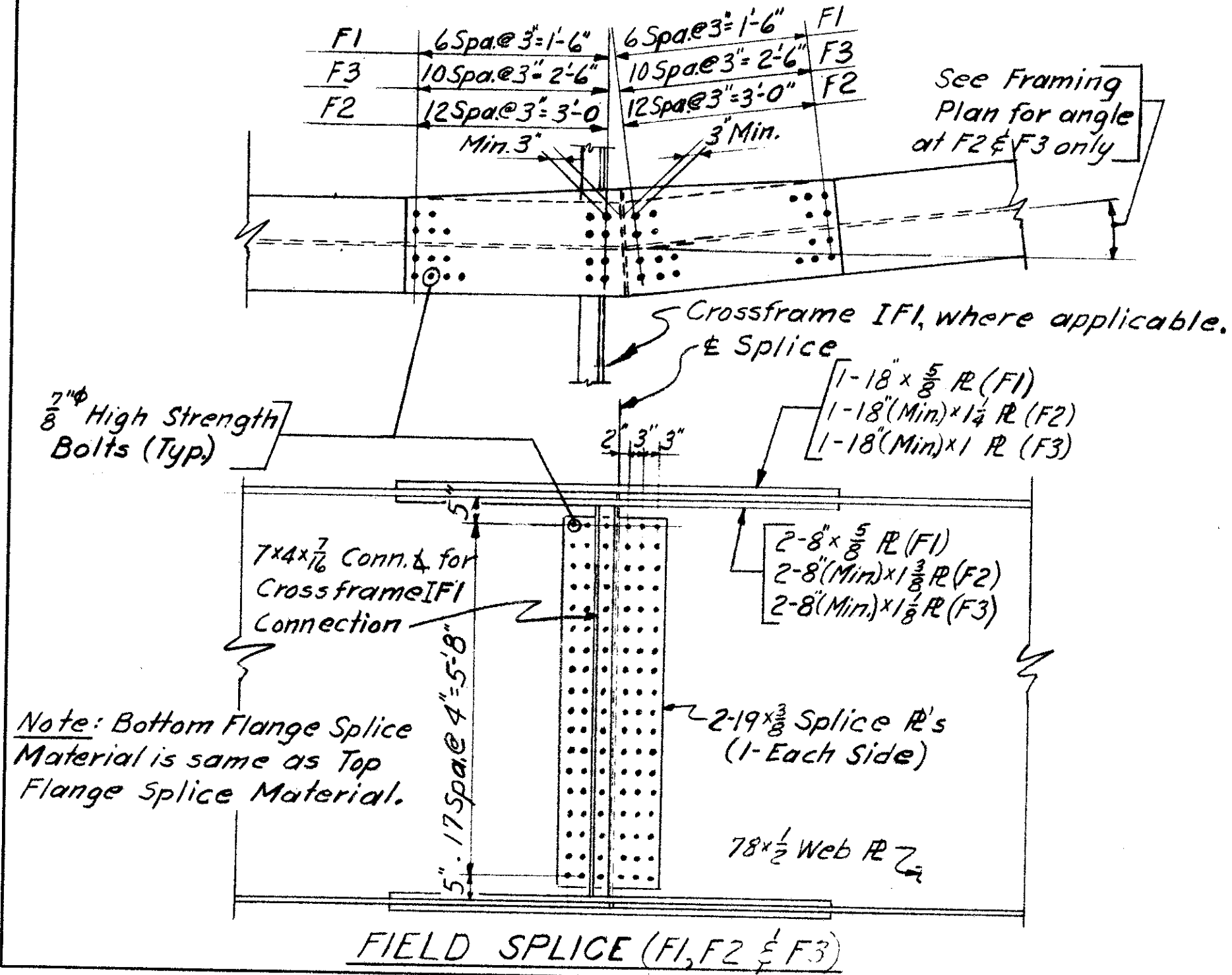


Gir. Lgth.	A thru. E	F thru. K
X	37'-0"	33'-0"
Y	18'-0"	23'-0"

R.No.	1	2	3
Girder A,B,C,D,E	18x1/8	18x1/8	18x2/8
F,G,H,J,K	18x1/8	18x2/8	18x1/8

Girder Dimension	A	B	C	D	E	F	G	H	J	K
a	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"	85'-1 1/2"
b	41'-7 3/8"	41'-1 1/8"	40'-7 3/8"	40'-1 1/2"	39'-7 3/8"	39'-4 3/8"	38'-11 1/2"	38'-5 1/2"	37'-11 3/8"	37'-6 3/4"
c	126'-8 1/2"	126'-2 1/2"	125'-8 1/2"	125'-2 3/8"	124'-8 3/8"	124'-5 3/8"	124'-0 3/8"	123'-6 3/8"	123'-1 1/2"	122'-7 3/8"
d	32'-4 1/2"	32'-10 1/8"	33'-4 1/2"	33'-10 1/2"	34'-4 3/8"	34'-7 3/8"	35'-0 3/8"	35'-6 3/8"	36'-0 3/8"	36'-5 3/8"
e	127'-5 3/8"	123'-10 3/8"	121'-0 1/2"	118'-7 3/8"	116'-5 3/8"	113'-0 3/8"	113'-10 3/8"	112'-8 3/8"	112'-2 1/2"	113'-0 3/8"
f	157'-9 3/8"	156'-9 3/8"	154'-4 3/8"	152'-6 3/8"	150'-10 3/8"	147'-7 3/8"	148'-11 1/2"	148'-3 3/8"	148'-2 1/2"	149'-6 3/8"

GIRDER SPAN	A		B		C		D		E		F		G		H		J		K																																					
	1	2	1	2	1	2	1	2	1	2	1	2	1	2	1	2	1	2																																						
LOCATION (See Diagram)	a	b	c	d	e	f	g	h	a	b	c	d	e	f	g	h	a	b	c	d	e	f	g	h	a	b	c	d	e	f	g	h	a	b	c	d	e	f	g	h	a	b	c	d	e	f	g	h								
Deflection due to Weight of Steel	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4	1/8	3/16	1/4	1/2	3/8	1/2	2/3	3/4
Deflection due to Remaining Dead Load	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1	3/8	9/16	3/4	5/8	1/2	3/4	5/8	1
Convexity & Super-elevation Transition	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16	2/8	1/16	1/8	1/16	1/16	1/16	1/16	1/16
Sum of Deflection, Convexity & Super-elevation Transition	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8	1 1/8	2 1/8	2 1/8	1 1/2	2 1/8	2 1/8	2 1/8	2 1/8								
Required Camber	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8	1 3/8	2 3/8	2 3/8	1 5/8	2 3/8	2 3/8	2 3/8	2 3/8								



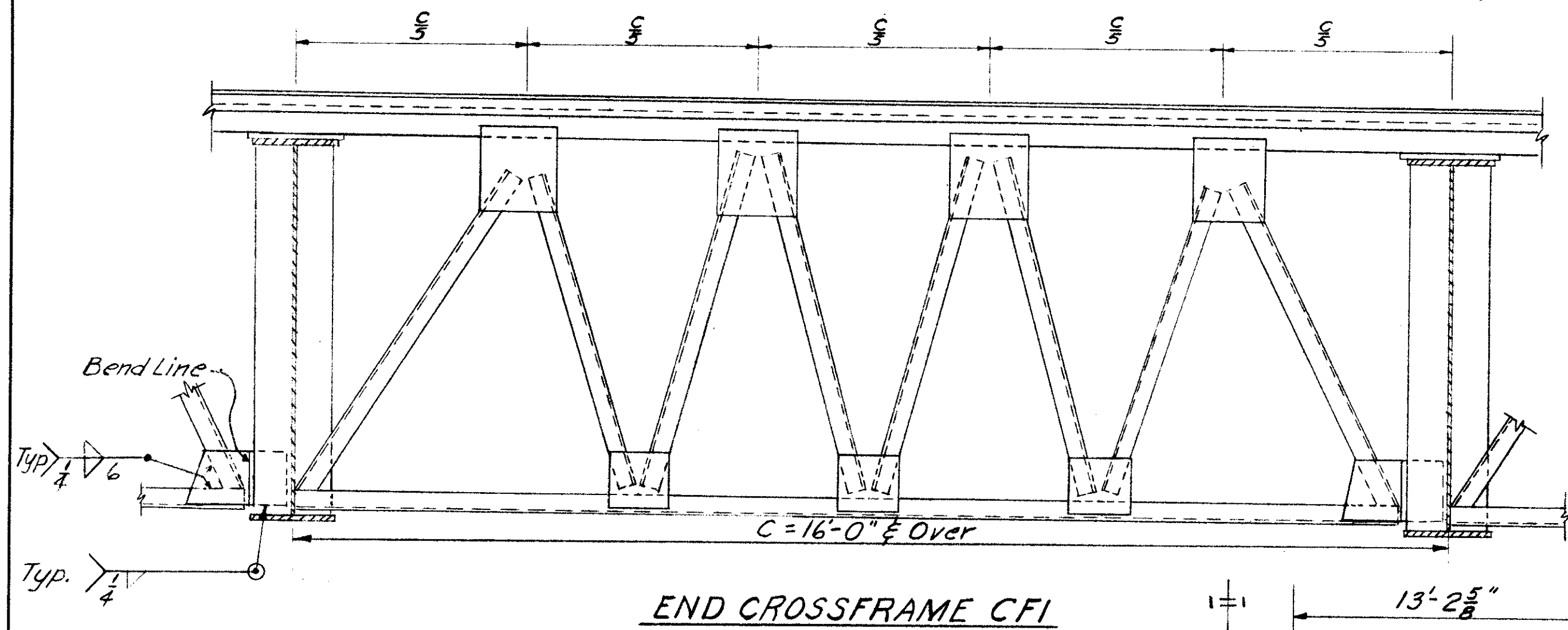
HAZLET & BERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

STRUCTURAL STEEL DETAILS

BRIDGE NO. HAM-71-0196

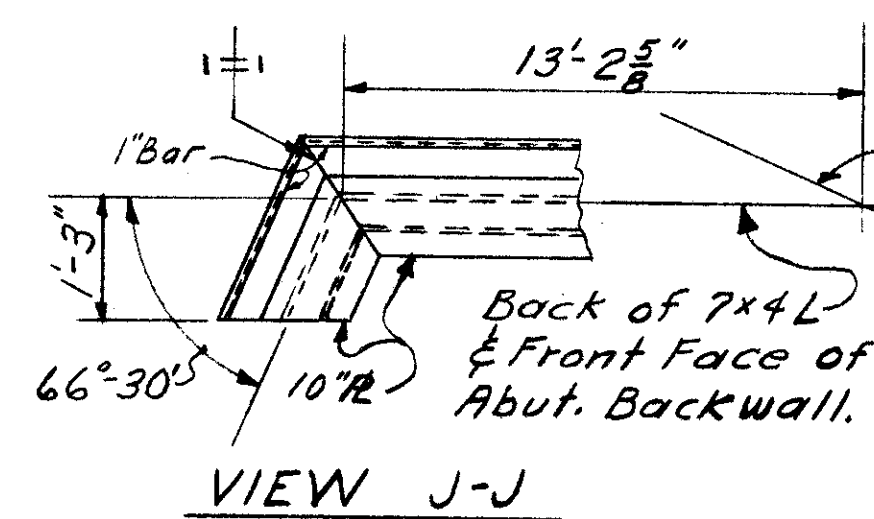
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	H.W.T.		M.J.C.	J.H.O.	
			5-17-35	5/20/65	

For details not shown see Std. Drwg. No. FSB-1-62.

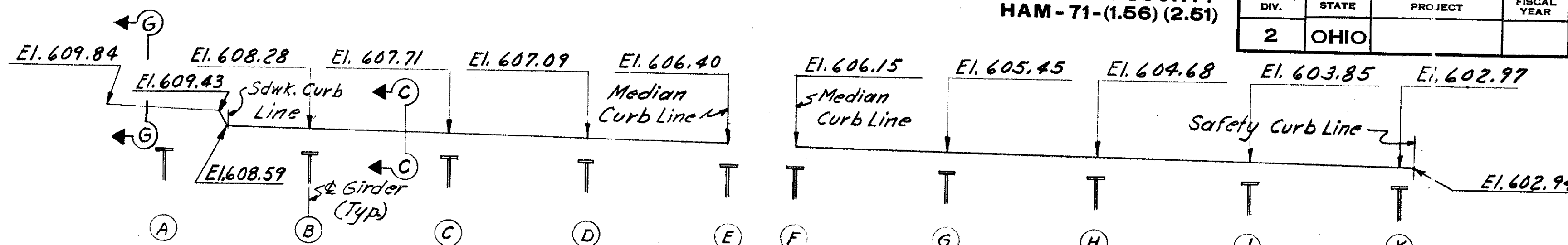


END CROSSFRAME CF1

Note: Unless otherwise shown all weld dimensions & member sizes for end crossframe CF1 & CF2 shall be as shown on Standard Drawing SD-1-63, Sheet #2.

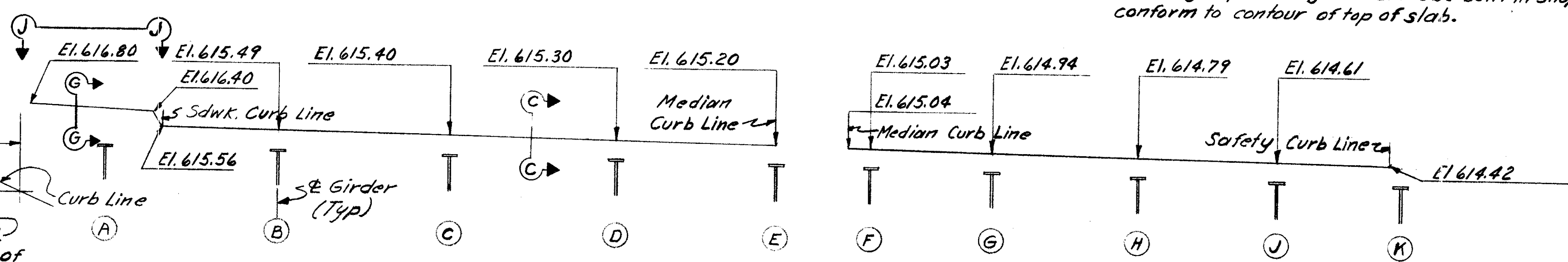


VIEW J-J

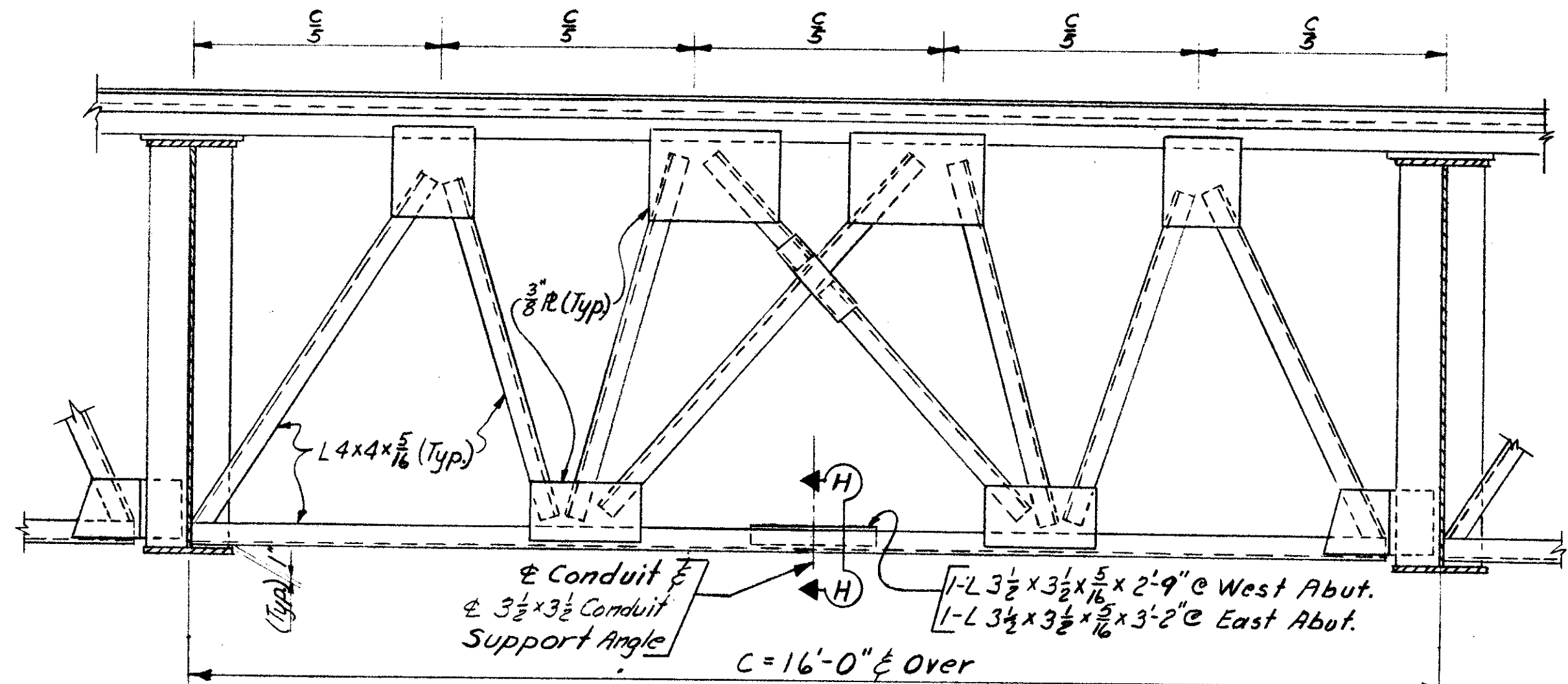


EXPANSION JOINT CROSS SECTION @ WEST ABUT.

Note: Elevations given are at the intersection of girder or curb line and expansion joint (shown on Section C-C)
Roadway expansion joints are to be bent in shop to conform to contour of top of slab.

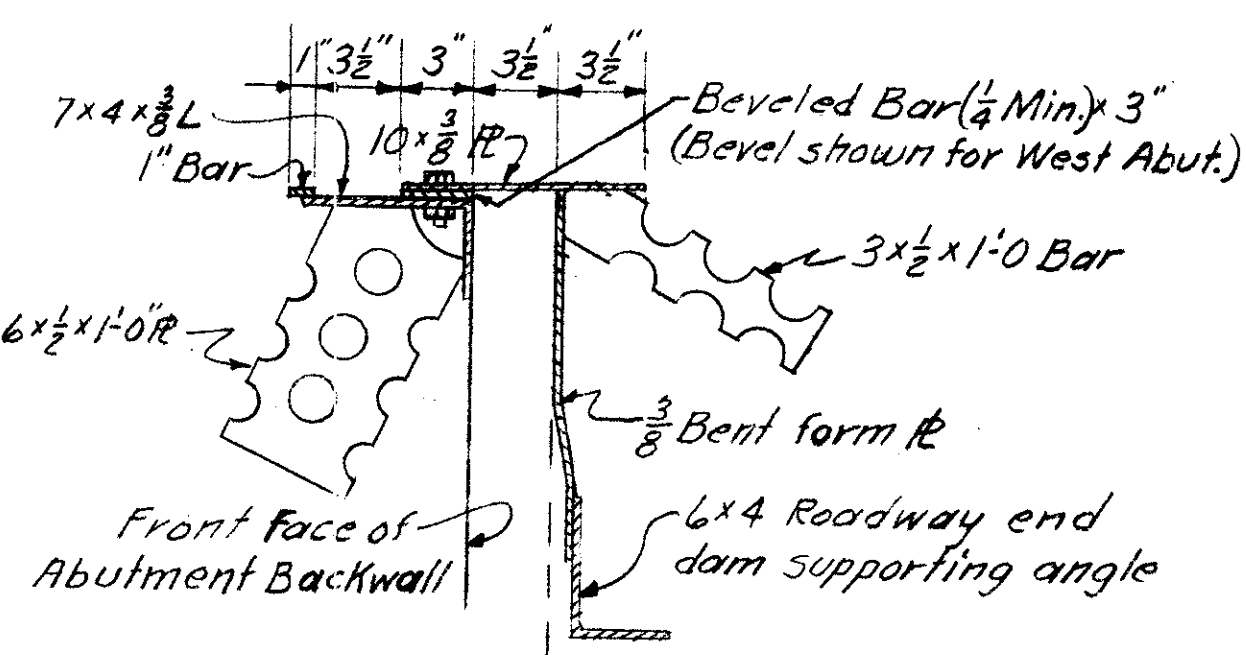


EXPANSION JOINT CROSS SECTION @ EAST ABUT.

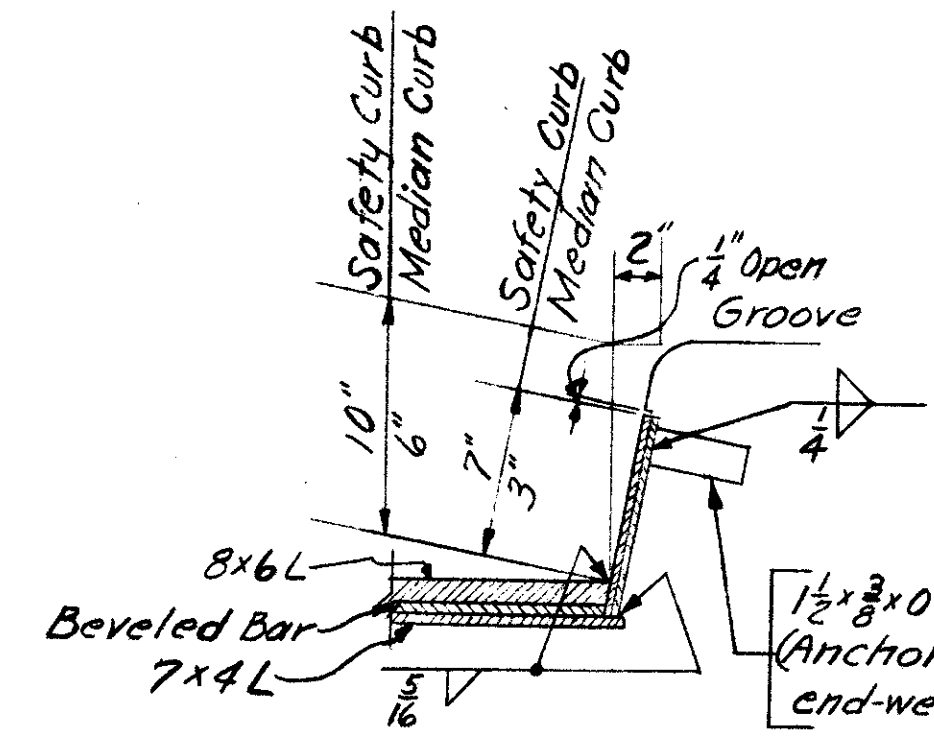


END CROSSFRAME CF2

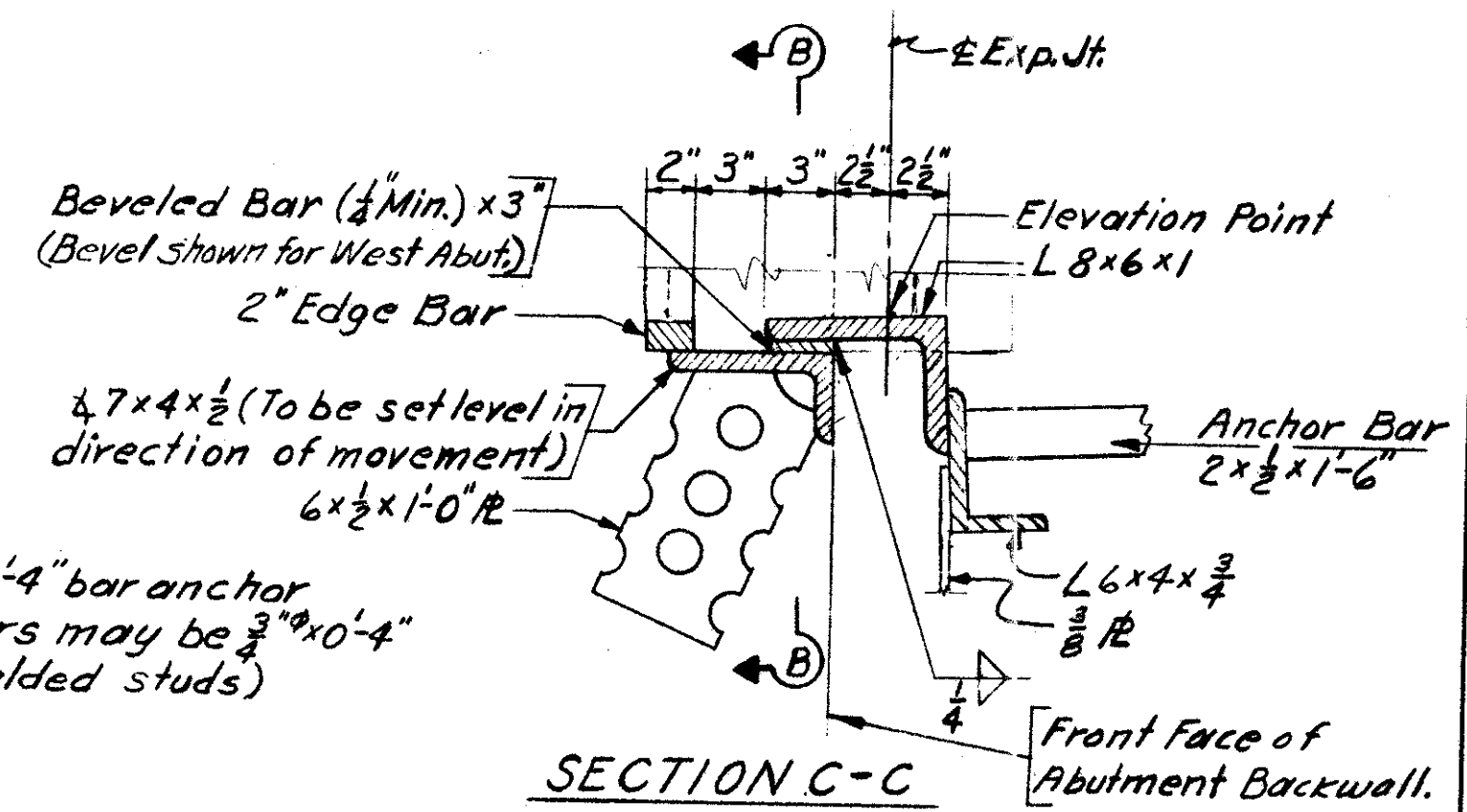
Provide 1/16" holes for 3/4" bolts in crossframe angles and stiffener plates, which are to be field connected before welding. If 3/8" bolts are to be left in place they shall be tack welded.



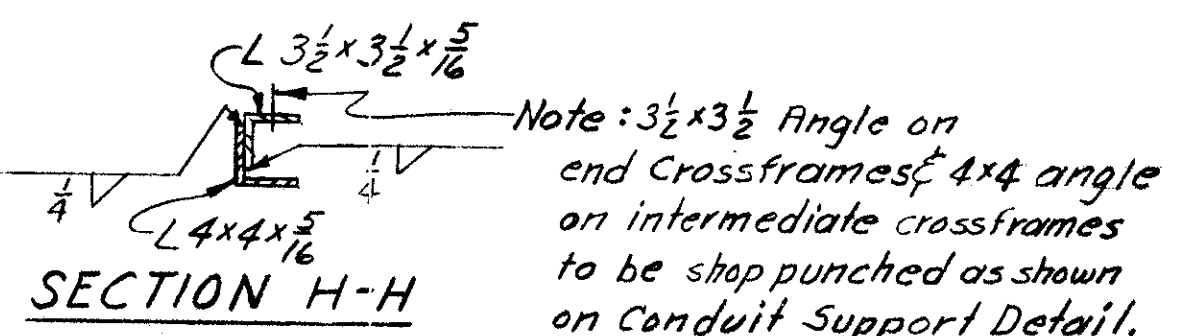
SECTION G-G



SECTION B-B
(Safety Curbs or Median Curbs)



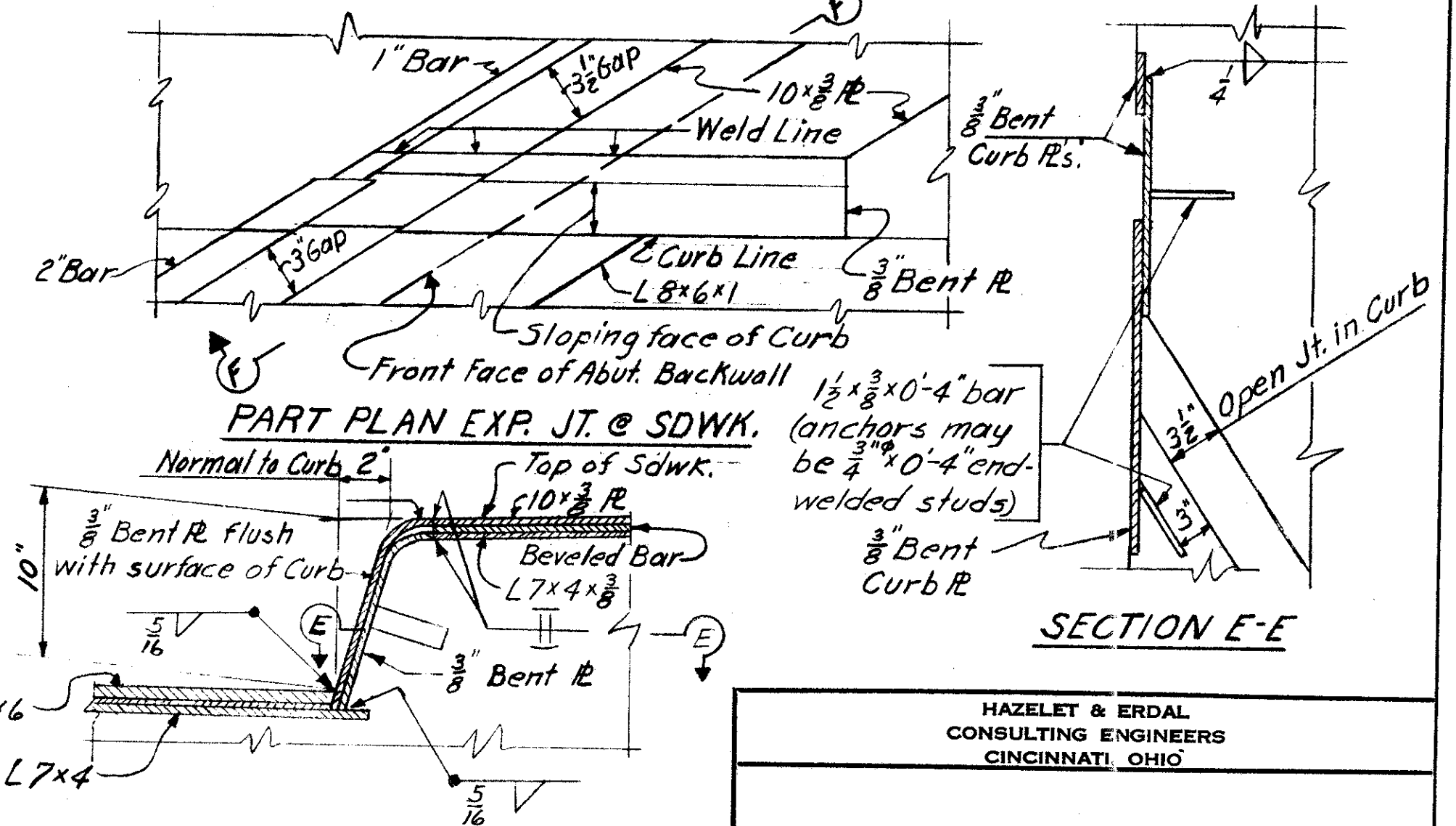
SECTION C-C



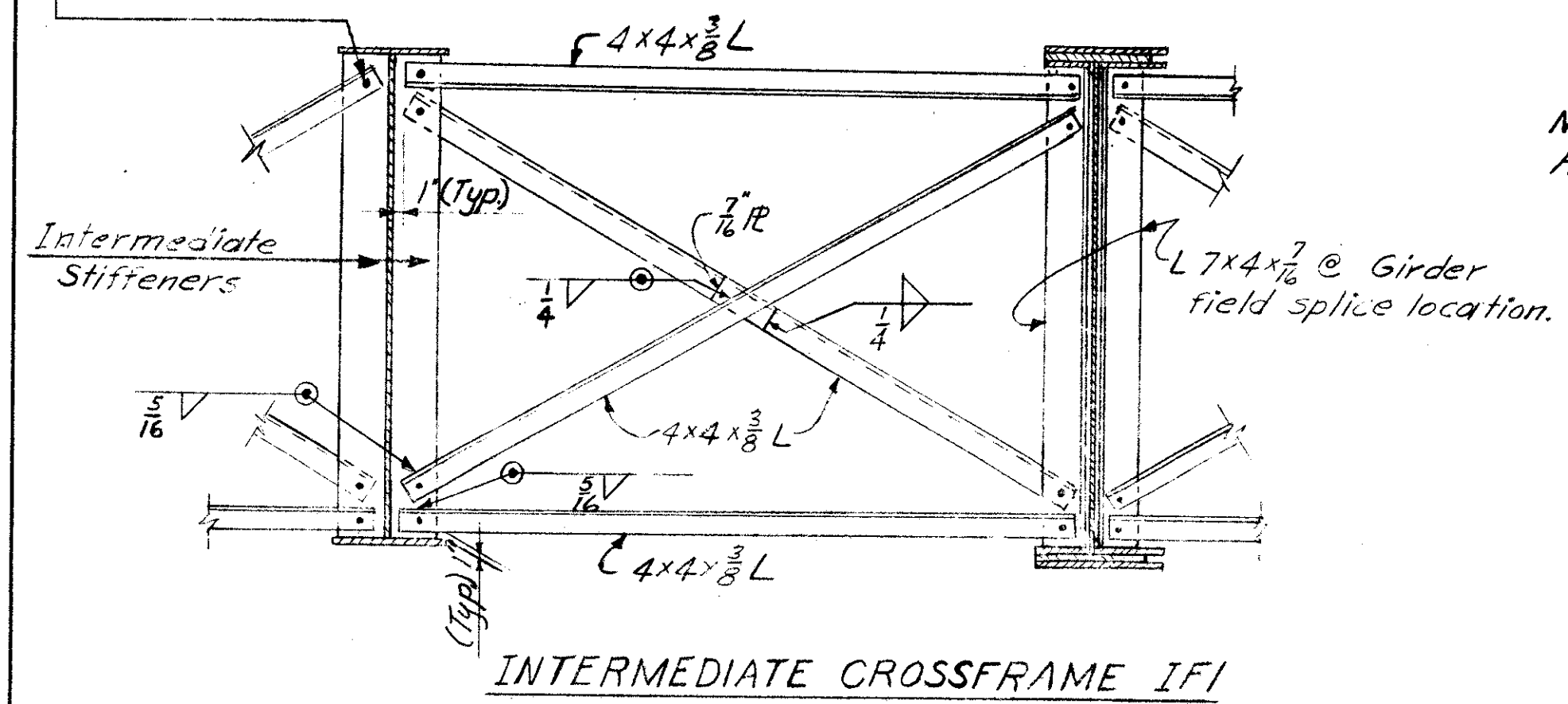
SECTION H-H

Note: 3 1/2 x 3 1/2 Angle on end crossframes & 4x4 angle on intermediate crossframes to be shop punched as shown on Conduit Support Detail.

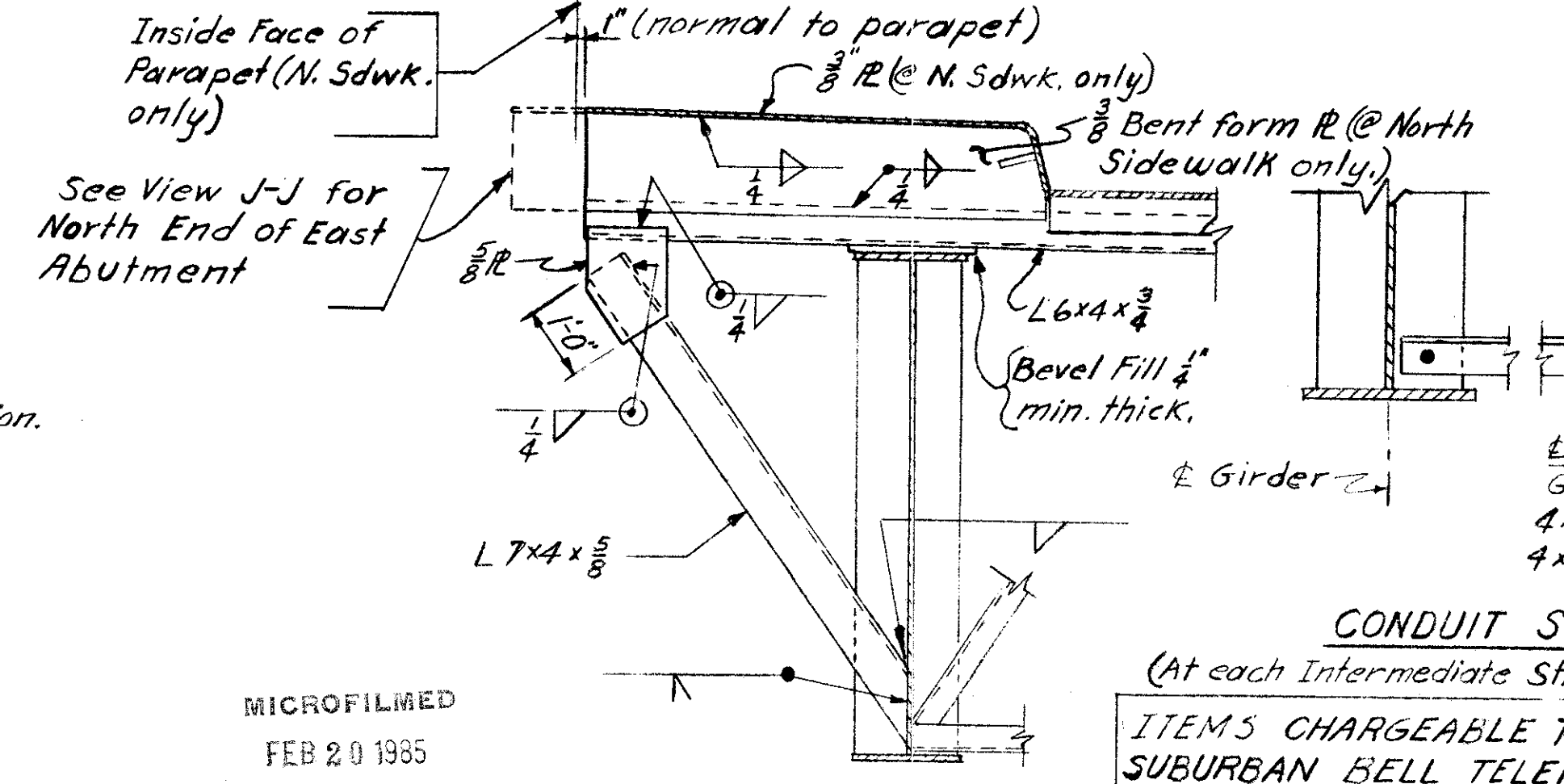
1 1/2" Slots Parallel to & of Conduits in Bays 3, & 4
3/4" holes in Bays 2 & 4



SECTION F-F



INTERMEDIATE CROSSFRAME IF1



TYPICAL BRACKET DETAIL

CONDUIT SUPPORT DETAIL

(At each Intermediate Stiffener Between Crossframes)

ITEMS CHARGEABLE TO CINCINNATI AND SUBURBAN BELL TELEPHONE CO.

Item	Unit	Description	Amount
5-7	Lbs.	Structural Steel	3600

ITEMS CHARGEABLE TO CINCINNATI GAS & ELECTRIC CO.

Item	Unit	Description	Amount
5-7	Lbs.	Structural Steel	8550

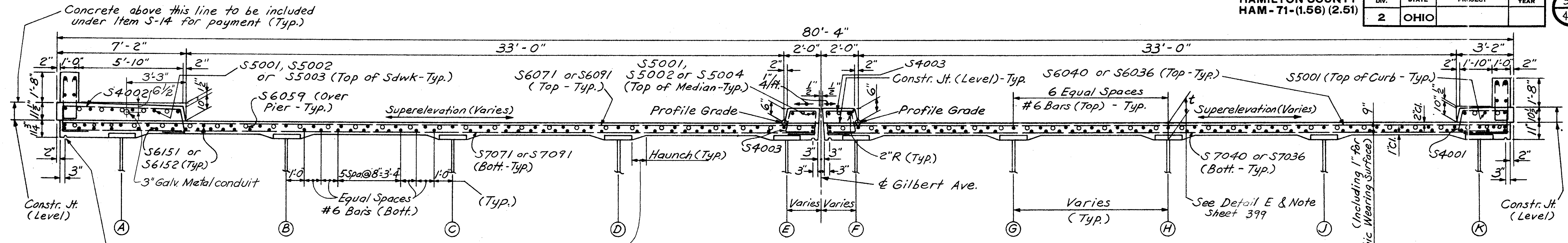
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HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

STRUCTURAL STEEL DETAILS

BRIDGE NO. HAM-71-0196

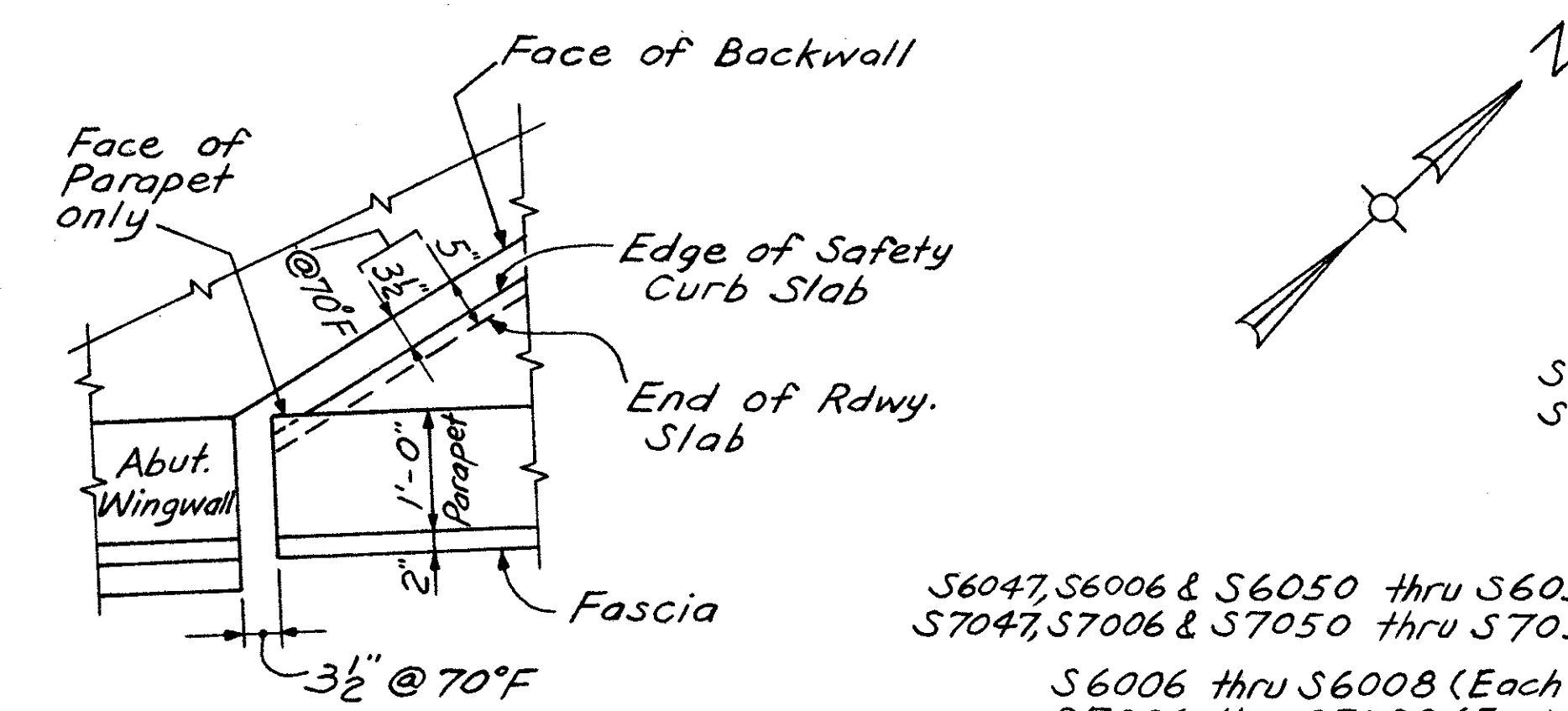
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	H.W.T.		W.J.Z. 3-17-65	J.H.O. 5/20/65	2-7-66



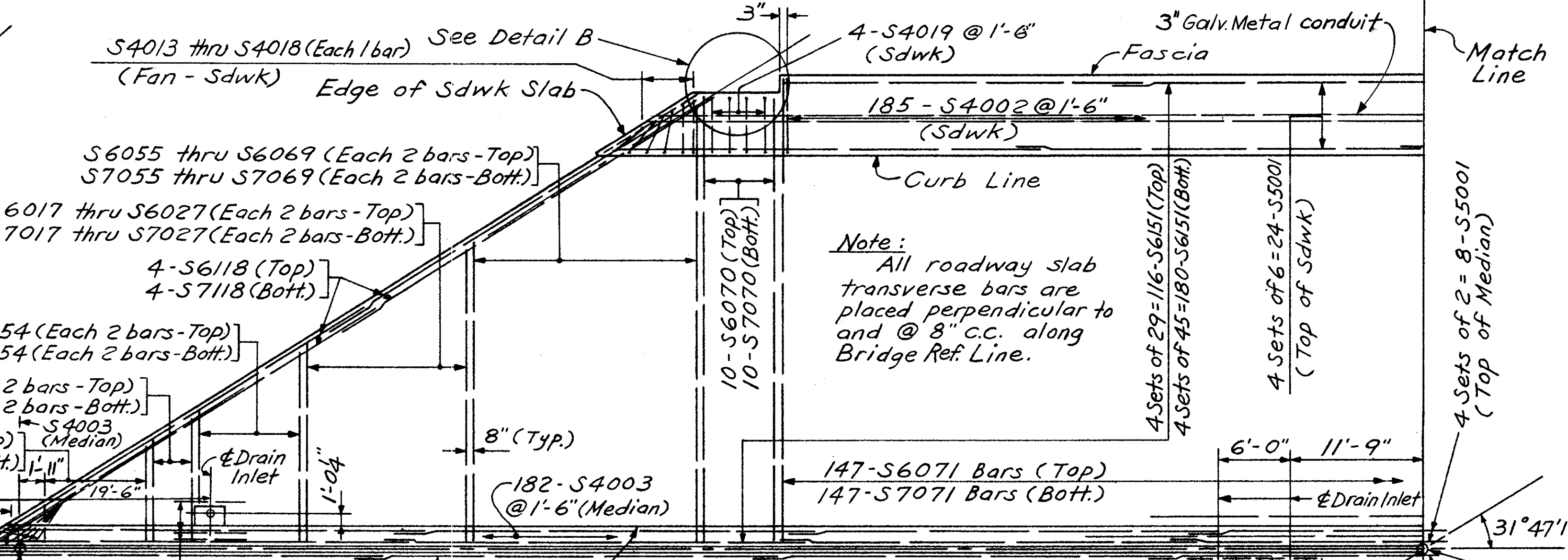
A typical haunch width of 9" shall be used for computing quantity of concrete. However, the haunch width may vary between 6" and 12" provided that the slope shall be not more than 1:4 for a haunch less than 9" wide.

3" Galv. Metal conduits to be located in sdwk and safety curb see Lighting Plan 5h No 206.

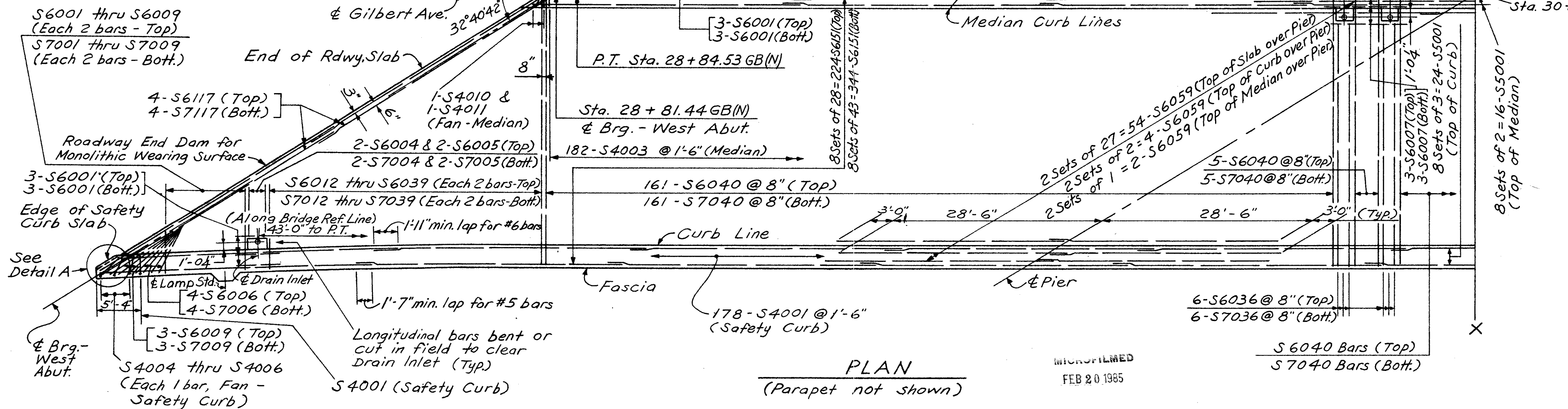
TRANSVERSE SECTION
(Constant Width Portion of Bridge - Looking East)



DETAIL A



DETAIL B



PLAN
(Parapet not shown)

Notes:
For Drainage Details, see Sheet 400
For Lighting Details, see Sheet 215.
For Railing Details and spacing of bars in parapet see Sheet 400
Work this drawing with Lighting Plan 5h. No 206
For conduit expansion detail at abutments see Sh No. 214.

Work this sheet with Sheet 399

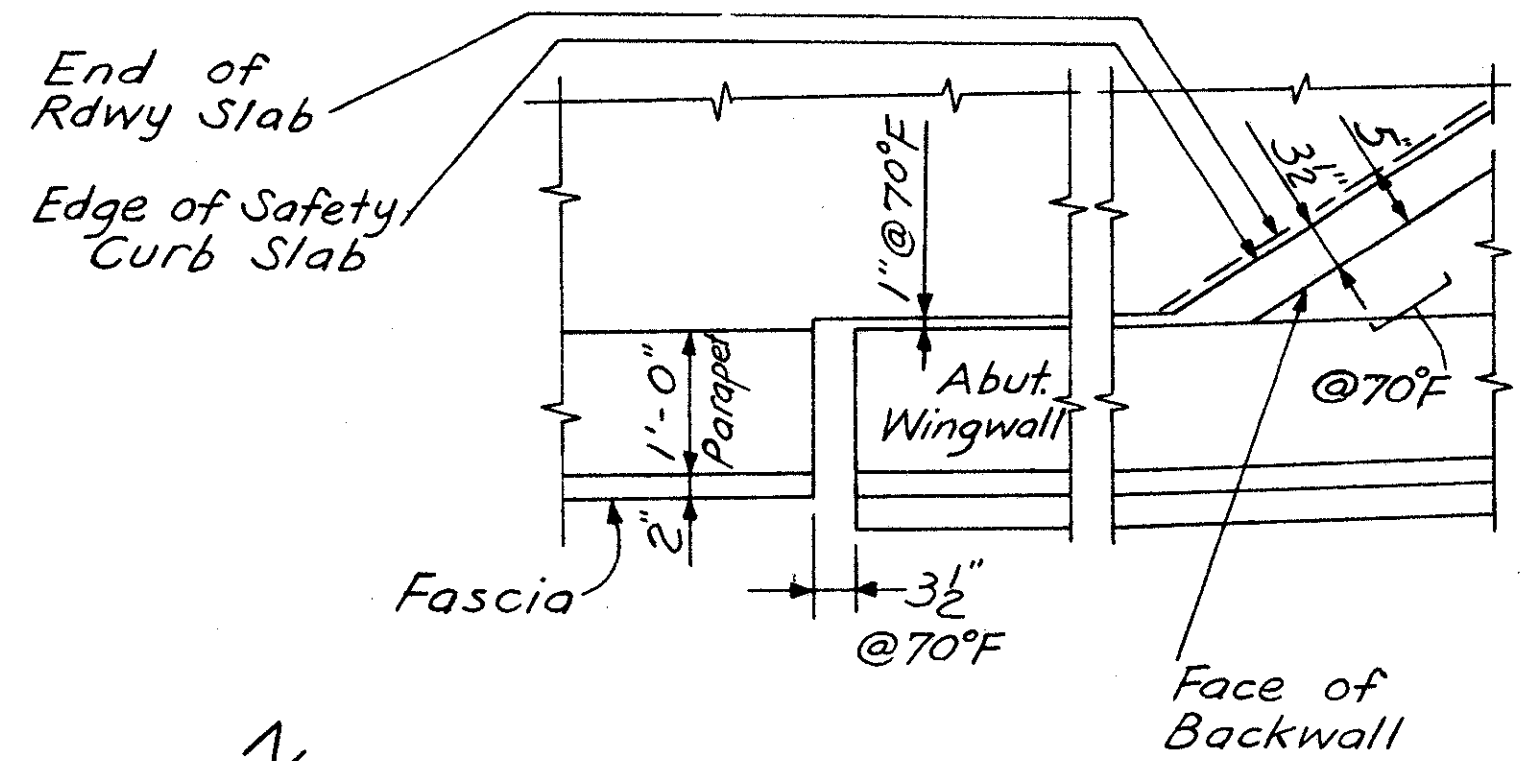
HAZELE & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SUPERSTRUCTURE DETAILS
BRIDGE NO. HAM - 71-0196

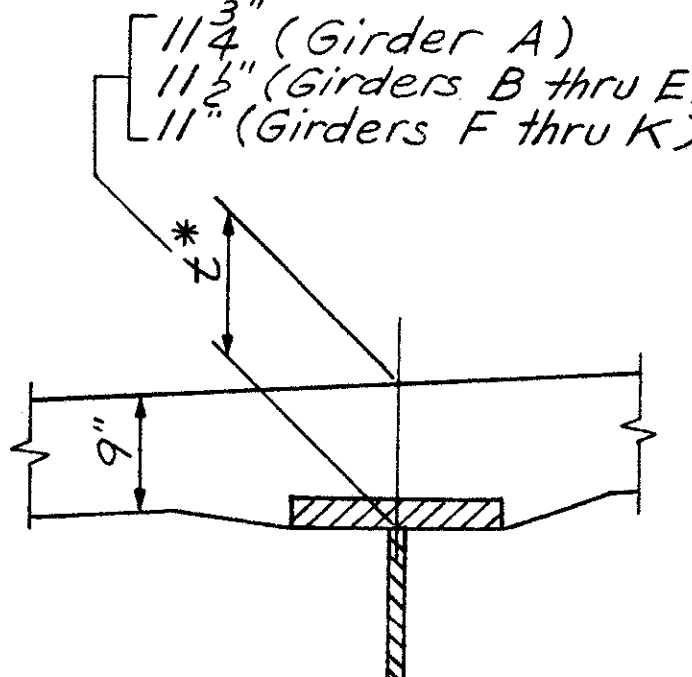
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	GW		JHO 3-17-65	JHO 5/20/65	

MICROFILMED
FEB 20 1985

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		



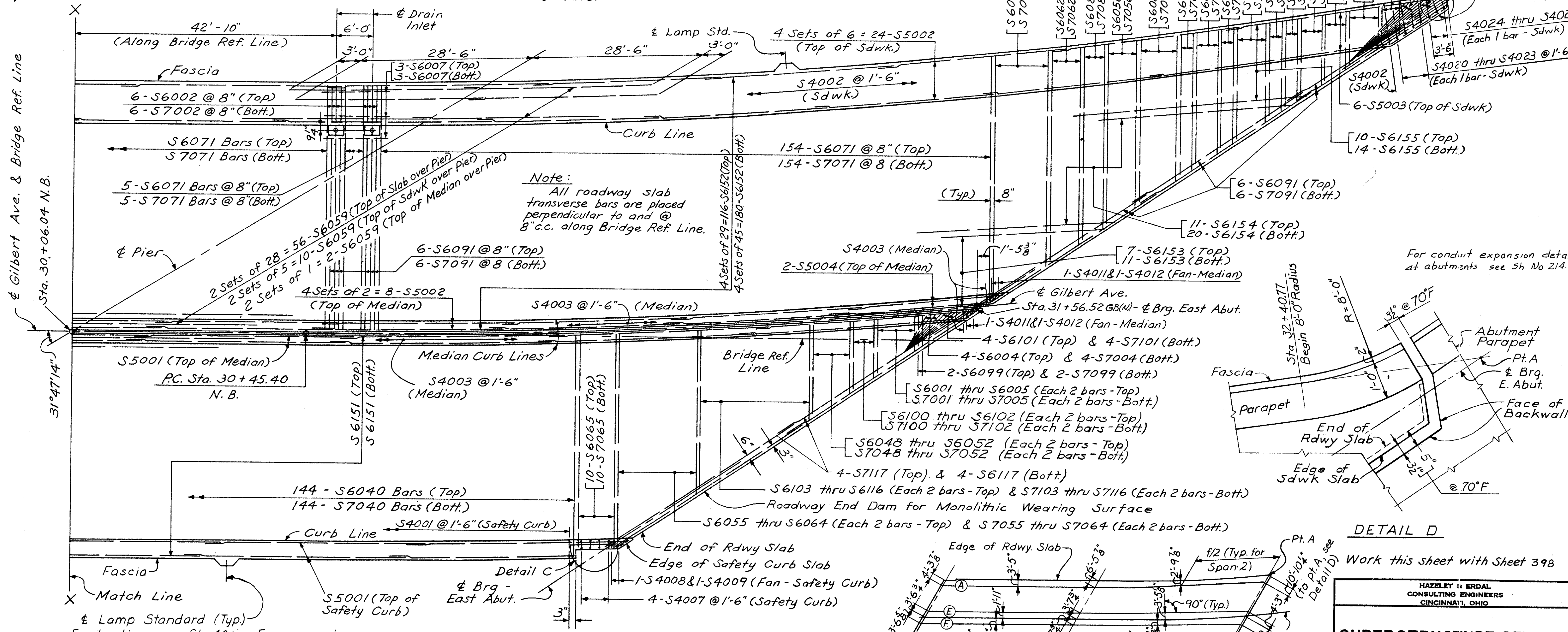
DETAIL C



DETAIL E

* This is the nominal dimension. The quantity of deck concrete to be paid for, shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade. Deduction shall be made for volume of encased steel plates as per Sec. S-1.25 of the Construction and Material Specification.

Note: Slab thickness includes 1" monolithic wearing surface.

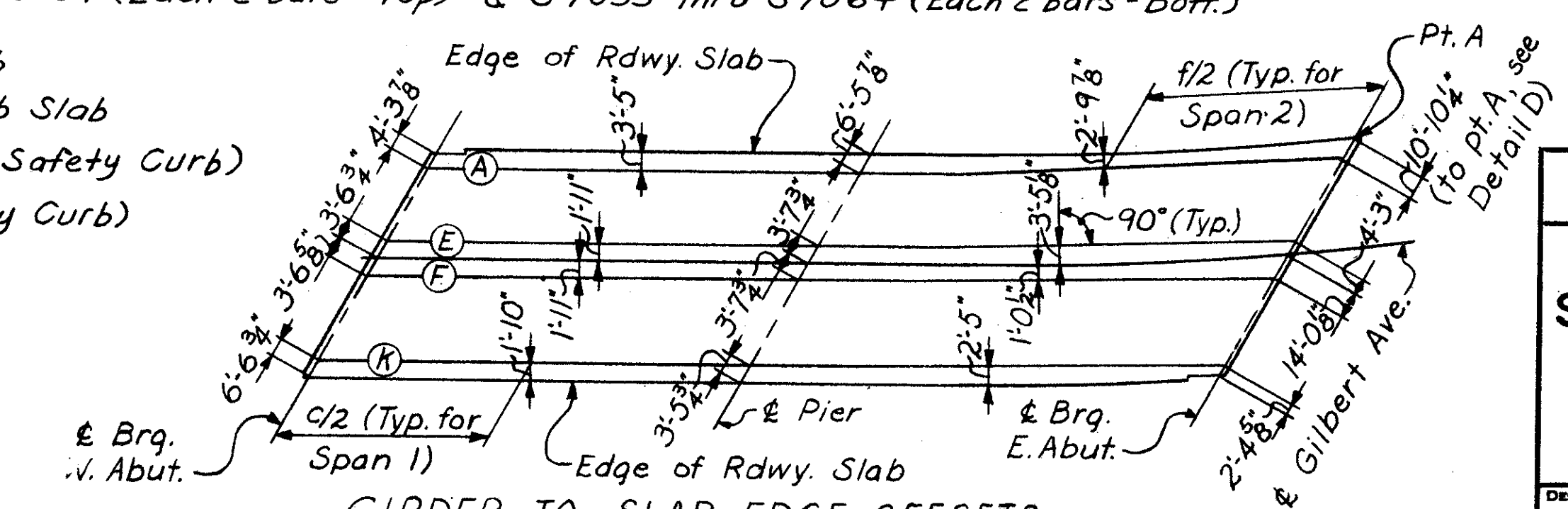


PLAN

(Parapet not shown)

(See Sh. 396 for girder lengths c and f)

GIRDER TO SLAB EDGE OFFSETS



DETAIL D

Work this sheet with Sheet 398

HAZELT & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SUPERSTRUCTURE DETAILS
BRIDGE NO. HAM - 71-0196

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	GW		D.W.	JHO 3-17-65

± Gilbert Ave. & Bridge Ref. Line
Sta. 30+06.04 N.B.

± Lamp Standard (Typ.)
For location, see Sh. 400. For support details, see Det. C & Sec. AA of Structure HAM-71-0203, Sh. 413 For lighting details not shown, see Sh. 215.

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FEB 20 1965

Note:
All roadway slab transverse bars are placed perpendicular to and @ 8" c.c. along Bridge Ref. Line.

For conduit expansion detail at abutments see Sh. No 214.

Detail D

Edge of Sdwk Slab

S4024 thru S4026 (Each 1 bar - Sdwk)

S4020 thru S4023 @ 1'-6" (Each 1 bar - Sdwk)

S4002 (Sdwk)

6-S5003 (Top of Sdwk)

[10-S6155 (Top)]

[14-S6155 (Bott.)]

[6-S6091 (Top)]

[6-S7091 (Bott.)]

[11-S6154 (Top)]

[20-S6154 (Bott.)]

[7-S6153 (Top)]

[11-S6153 (Bott.)]

1-S4011 & 1-S4012 (Fan-Median)

2-S5004 (Top of Median)

S4003 (Median)

S4002 @ 1'-6" (Sdwk)

6-S6091 @ 8" (Top)

6-S7091 @ 8" (Bott.)

4-S6101 (Top) & 4-S7101 (Bott.)

4-S6004 (Top) & 4-S7004 (Bott.)

2-S6099 (Top) & 2-S7099 (Bott.)

[S6001 thru S6005 (Each 2 bars - Top)]

[S7001 thru S7005 (Each 2 bars - Bott.)]

[S6100 thru S6102 (Each 2 bars - Top)]

[S7100 thru S7102 (Each 2 bars - Bott.)]

[S6048 thru S6052 (Each 2 bars - Top)]

[S7048 thru S7052 (Each 2 bars - Bott.)]

4-S7117 (Top) & 4-S6117 (Bott.)

S6103 thru S6116 (Each 2 bars - Top) & S7103 thru S7116 (Each 2 bars - Bott.)

Roadway End Dam for Monolithic Wearing Surface

S6055 thru S6064 (Each 2 bars - Top) & S7055 thru S7064 (Each 2 bars - Bott.)

10-S6065 (Top)

10-S7065 (Bott.)

S4001 @ 1'-6" (Safety Curb)

1-S4008 & 1-S4009 (Fan - Safety Curb)

4-S4007 @ 1'-6" (Safety Curb)

End of Rdwy Slab

Edge of Safety Curb Slab

± Brq. East Abut.

± Brq. E. Abut.

± Pier

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

± Brq. E. Abut.

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± Brq. E. Abut.

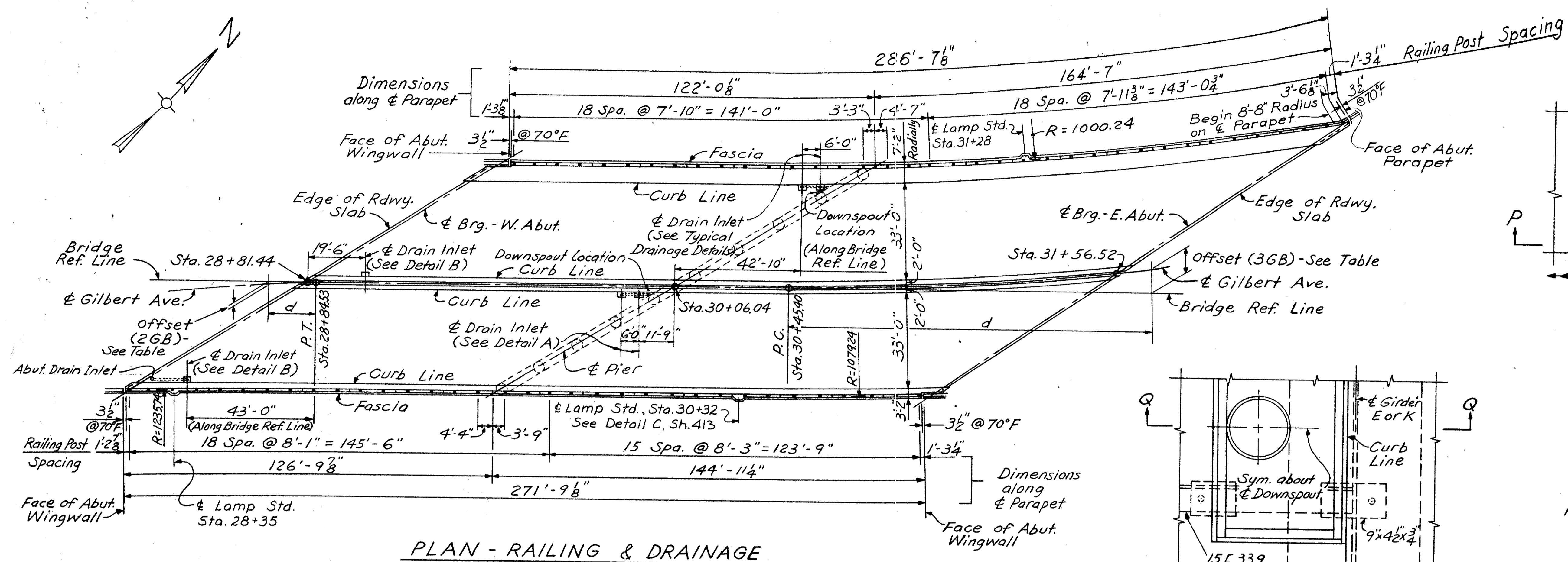
± Brq. E. Abut.

± Brq. E. Abut.

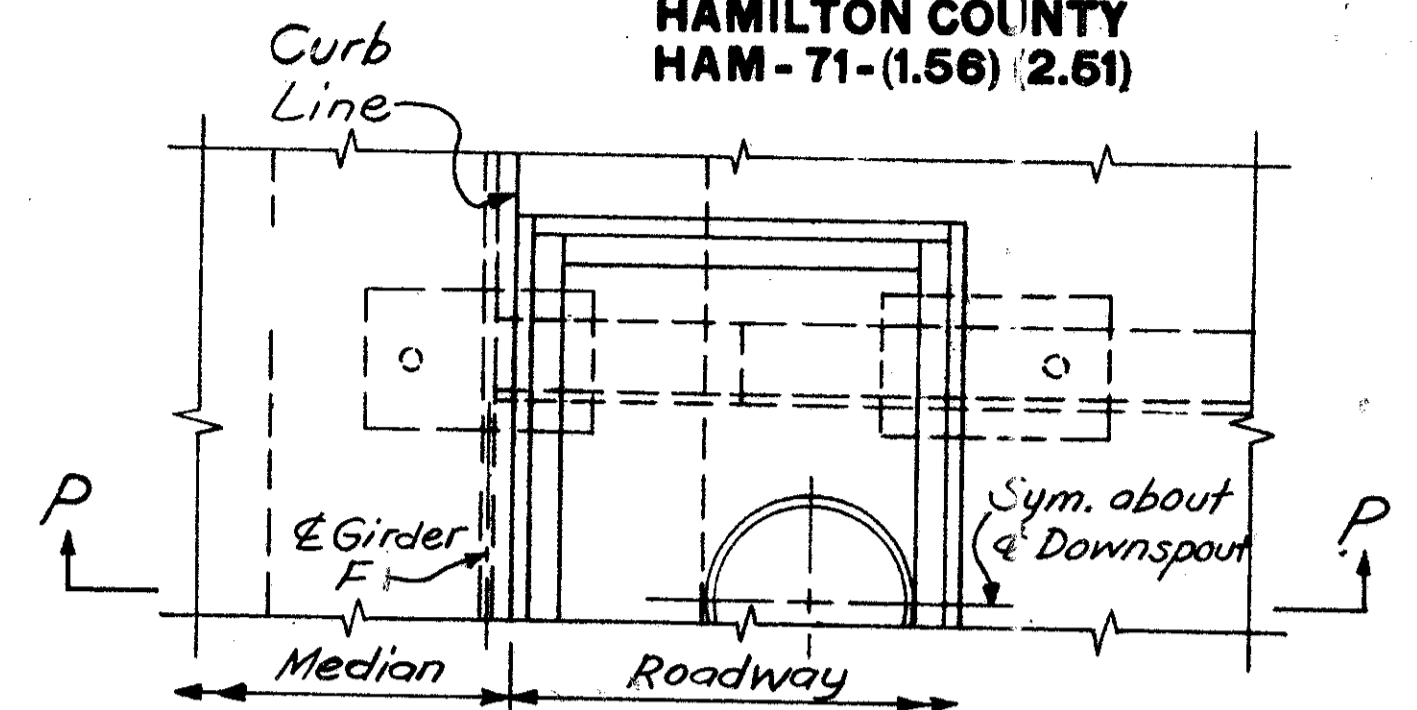
± Brq. E. Abut.

± Brq. E. Abut.

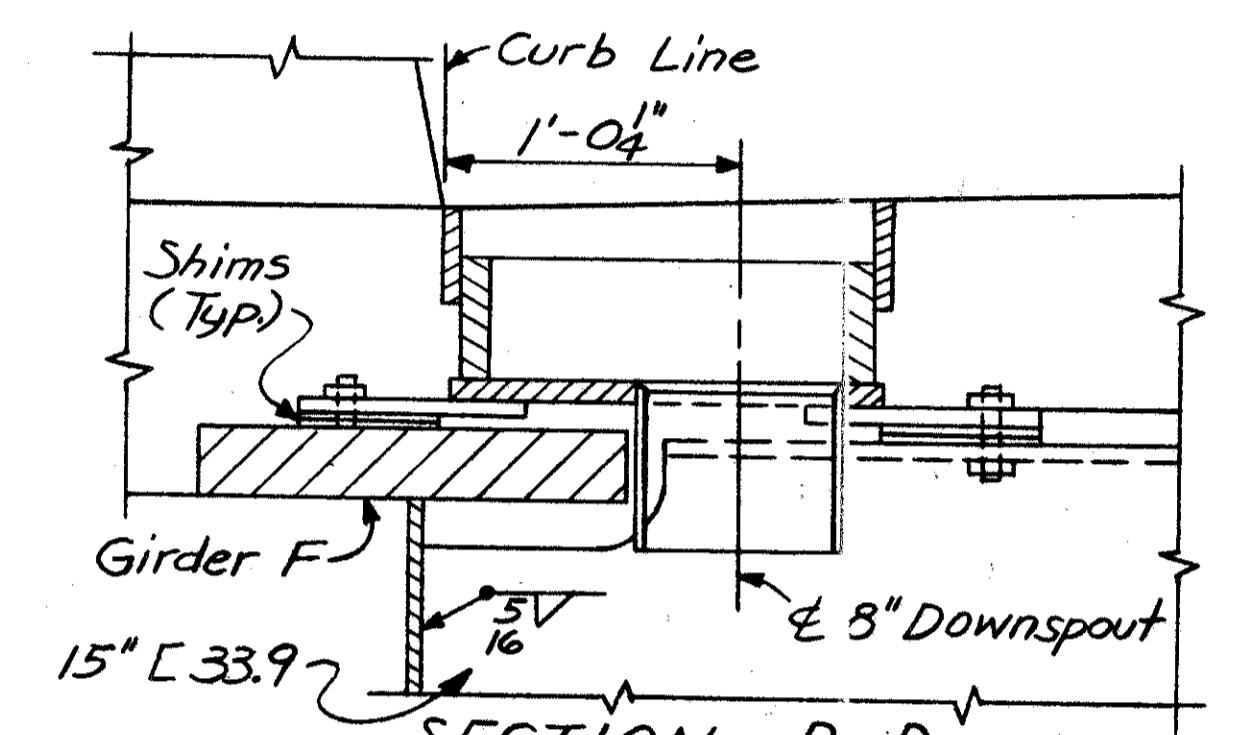
HAMILTON COUNTY
HAM - 71-(1.56) (2.51)



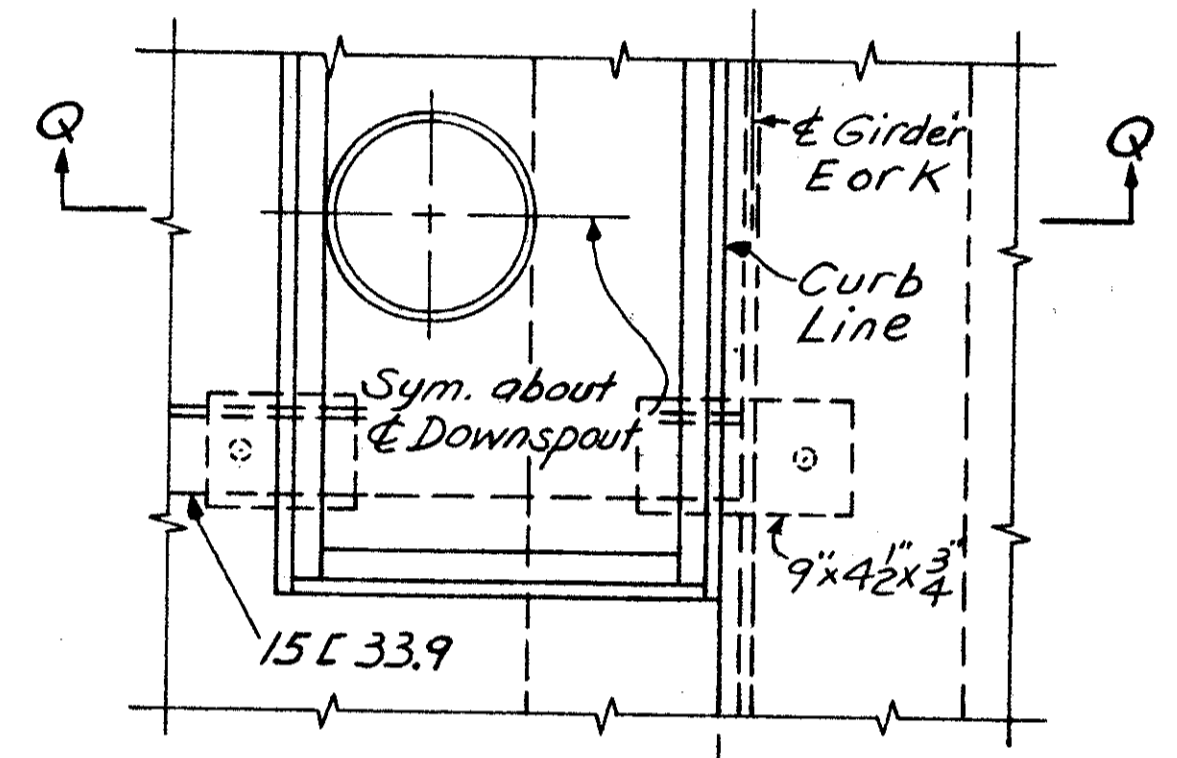
PLAN - RAILING & DRAINAGE



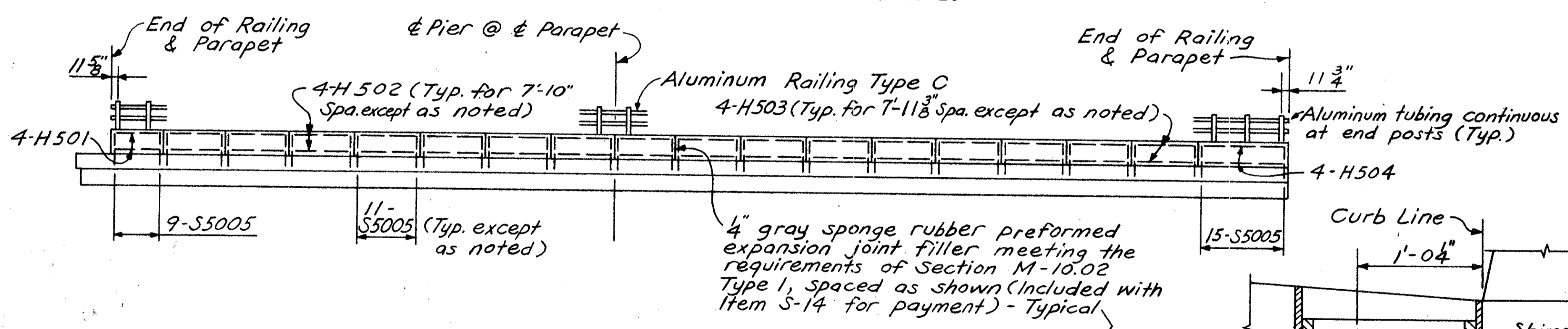
DETAIL A



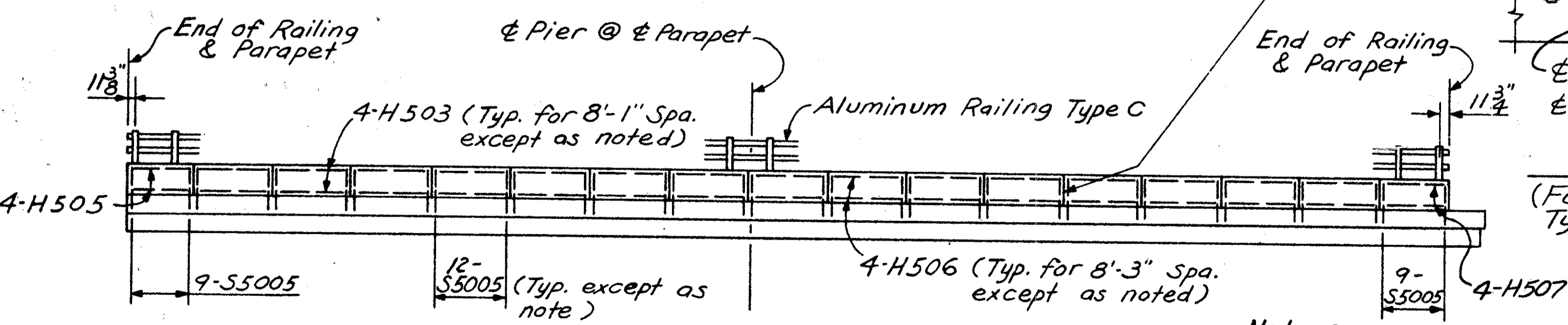
SECTION P-P
(For details not shown, see Typical Drainage Details)



DETAIL B

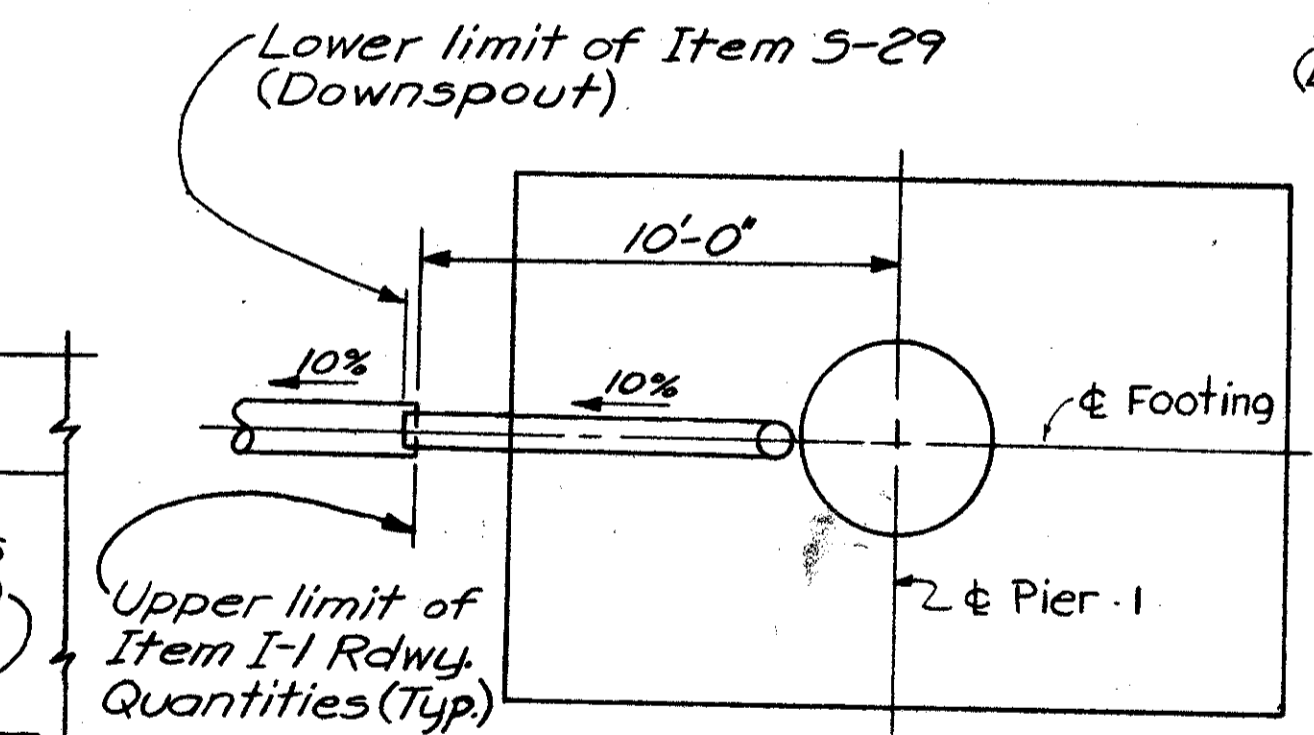


ELEVATION - NORTH RAILING

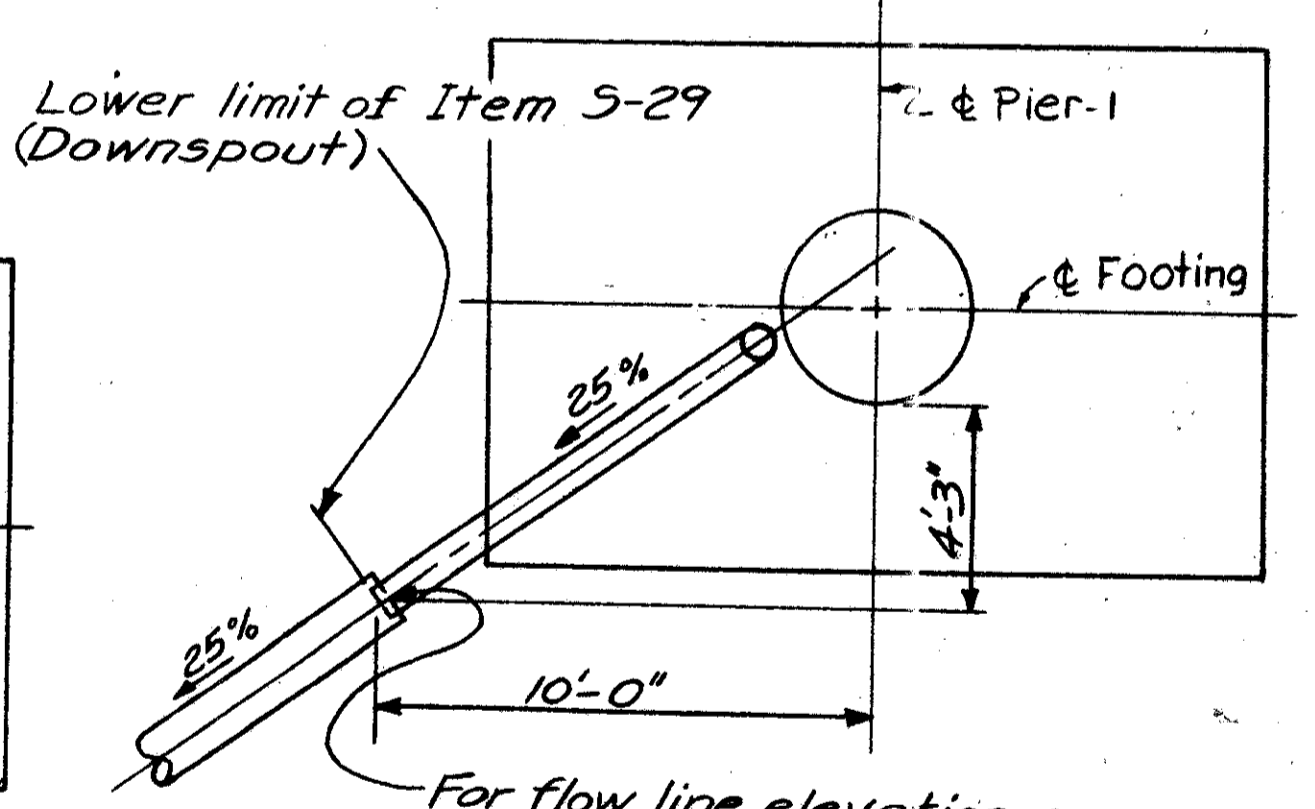


ELEVATION - SOUTH RAILING

Notes:
For railing details not shown, see Standard Drawing No. AR-1-57
Aluminum tube railing shall be bent to the curvature of the $\&$ of parapet.



PLAN OF LIMITS OF 5-29 & I-1 AT PIER NO. 1 FOR SOUTHBOUND ROADWAY



PLAN OF LIMITS OF 5-29 & I-1 AT PIER NO. 1 FOR NORTHBOUND ROADWAY

Location	@ $\&$ Brg. Abut.	@ Distance d from P.T. or P.C.							
		25'	50'	75'	100'	125'	150'	175'	200'
Offset Curve 2GB(N.B.)	16"	3"	11 3/4"	2'-2 1/2"	3'-11 1/4"	-	-	-	-
Offset Curve 3GB(N.B.)	5'-11"	3 3/8"	1'-2 3/8"	2'-8 3/8"	4'-9 3/4"	7'-6 3/8"	10'-10 1/4"	14'-9 3/8"	19'-4 1/2"

TABLE OF OFFSETS

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FEB 20 1965

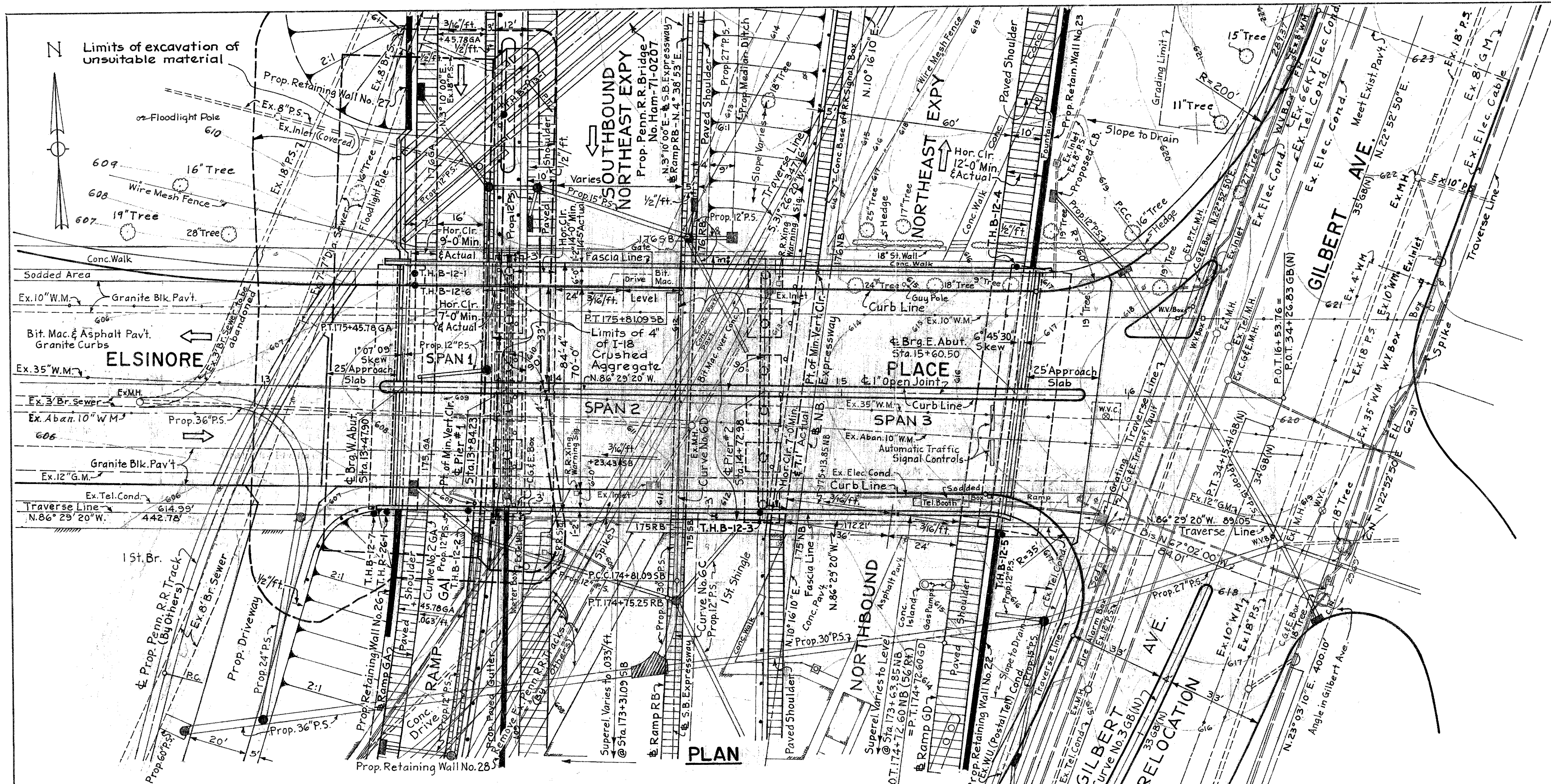
Work this sheet with Sheets 318 & 399

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CONSULTING ENGINEERS
CINCINNATI, OHIO

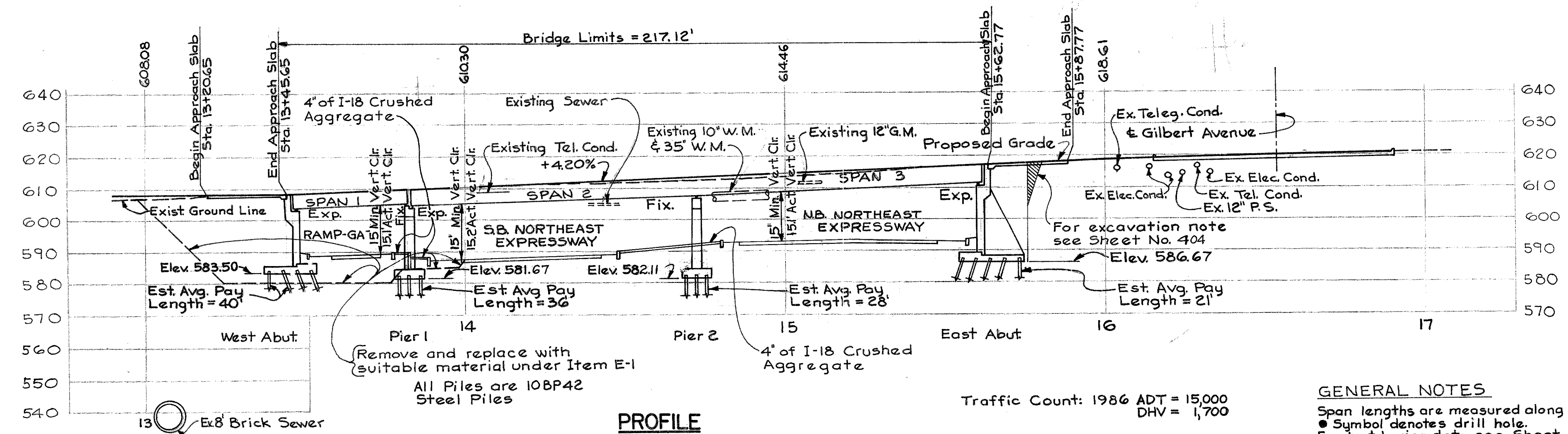
RAILING DETAILS
BRIDGE NO. HAM - 71-0196

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED
	GW		D.M. 3-17-65	

HAMILTON COUNTY
HAM-71-(156)(251)



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PROPOSED STRUCTURE
 Type: Simple Span Steel Beam (Span 1) and Continuous Plate Girder (Spans 2 & 3) with Reinforced Concrete Deck & Substructure
 Spans: 36'-4", 88'-9", 87'-6 1/4" 9/16" Bearings (See Note)
 Roadway: 33'-0" f/f curbs with 4'-0" median and two 6'-0" sidewalks.
 Skew: Varies, see Plan.
 Load frequency: CF = 2000(57)
 Wearing Surface: 1" Monolithic Concrete
 Approach Slab: AS-1-54 (25'-0" long)
 Alignment: Tangent
 Superelevation: Varies, see Plan

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SITE PLAN
BRIDGE No. HAM-71-0203
EL SINORE PLACE

H & E BRIDGE No. 12

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
JDC	JDC		N.A.B.	JHE 5/20/65	

Traffic Count: 1986 ADT = 15,000
DHV = 1,700

GENERAL NOTES
 Span lengths are measured along Base Line
 ● Symbol denotes drill hole.
 For test boring data, see Sheet 8, 9 & 11 of 13
 For Bench Marks see Sheet 40

HAMILTON COUNTY
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GENERAL NOTES

STEEL PILES: shall be driven with a hammer of not less than 11,000 ft. lbs. per blow to firm contact with rock or shale. If the length of penetration is approximately equal to the depth of rock and shale according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Section S-18.05 is not less than 50 tons per pile using an 11,000 ft. lb. hammer or 45 tons per pile using a 15,000 ft. lb. hammer. If the energy rating of the hammer is between the rating as shown above, the required formula capacity shall be determined by interpolation. The design of abutments and piers is based on 35 tons per pile.

DESIGN NOTES:
Design Loading CF = 2000 (57)
Concrete Class C - Basic unit stress 1333 psi.
Concrete Class E - Basic unit stress 1133 psi.
Structural Steel - ASTM A36 - Basic unit stress 20,000 psi. (ASTM A7 and A373 steel not permitted except for piling)
Reinforcing Steel - ASTM A15, A16, A160. Deformed Intermediate or Hard Grade. Basic unit stress 20,000 psi. Except spiral reinforcement may be plain structural grade with basic unit stress of 18,000 psi.

MACHINE FINISH OF CONCRETE: The concrete bridge deck shall be finished by the use of a finishing machine.

ADDITIONAL NOTES: For additional notes see Notes 1 thru 8, General Notes, Typical Drawing No. 443

EXCAVATION For the purpose of driving battered piles at the East Abutment (Shown hatched on Sheet No. 403) shall be included with "Unclassified Excavation" for payment.

EXCAVATION OF UNSUITABLE MATERIALS: After the unsuitable materials have been removed to elevation 580.0, at the West Abutment as shown on the Site Plan, the Engineer shall make a careful inspection to ascertain that all unsuitable materials have been removed to a stable subsoil. If necessary, the excavation limits shall be extended to remove additional unstable materials and be replaced with materials meeting the requirements of Section E-1 of the Specifications.

Embankment to replace unsuitable material shall be compacted to a minimum of 8" above the bottom of footing elevations before proceeding with structure excavation, (Item E-2, unclassified excavation) at the West abutment and Pier 1.

ESTIMATED QUANTITIES									
Item	Total	Unit	Descriptions	Super-structure	West Abutment	East Abutment	Pier 1	Pier 2	General
E-2	Lump	Sum	Cofferdams, cribs & sheeting						Lump
E-2	2208	Cu. Yds.	Unclassified Excavation		40	1845	52	351	
S-1	630	Cu. Yds.	Class "C" concrete, superstructure	630					
S-1	142	Cu. Yds.	Class "C" concrete, piers above footings				79.8	62.2	
S-1	399	Cu. Yds.	Class "E" concrete, abutments above footings		153.7	245.3			
S-1	541	Cu. Yds.	Class "E" concrete, footings		148.8	238.8	66.7	86.7	
S-3	42	Lin. Ft.	Waterproofing, premolded sealing strip		42				
S-3	8	Sq. Yds.	Type "B" waterproofing		8				
S-4	255,099	Lbs.	Reinforcing steel	120788	33532	49013	19981	31785	
S-7	565000	Lbs.	Structural Steel	565000					
S-8	565000	Lbs.	Field painting of structural steel	565000					
S-9	56	Sq. Ft.	1" Preformed expansion joint filler		51		5		
S-14	442	Lin. Ft.	Railing, type "C" (aluminum rail & supports, concrete parapet & end posts)	423	12	7			
S-16	Lump	Sum	First test pile						Lump
S-18	8307	Lin. Ft.	Steel piles 10 BP42	2400	2661	1728	1512		
S-25			Electric Lighting System (See Sh. No 204)						
S-29	291	Cu. Yds.	Porous backfill		134	157			
S-29	Lump	Sum	Drain inlet including supports and horizontal collector system	Lump					
S-29	108	Lin. Ft.	6" standard pipe downspout, wrought iron or hot-dip galvanized steel, including specials.				49	59	
S-29	96	Lin. Ft.	8" bituminous coated helical perforated corrugated metal pipe [Sec. M-6.4(h-c)] (Including specials)		48	48			
S-29	178	Lin. Ft.	8" bituminous coated helical perforated corrugated metal pipe [Sec. M-6.4(h-c)] (Including specials & sand)		86	92			
S-29	4	Lin. Ft.	12" reinforced concrete sewer pipe [Sec. M-6.6(a)]		4				
S-29	31	Lin. Ft.	8" bituminous coated helical corrugated metal pipe [Sec. M-6.4(h-c)] (Including specials)		31				
S-101	630	Each	Water-reducing set-retarding admixture	630					

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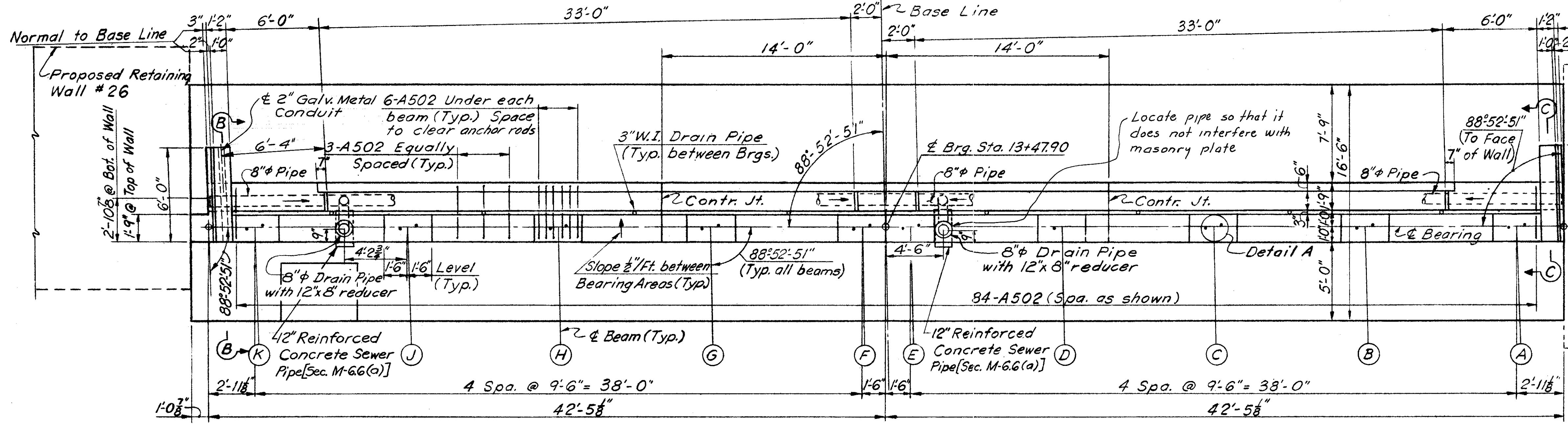
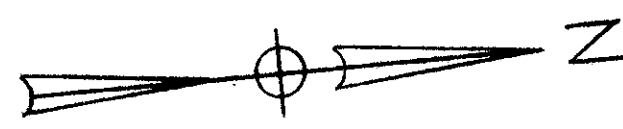
HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

ESTIMATED QUANTITIES & NOTES

BRIDGE NO. HAM - 71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	S.M.		W.J.Z.	JHO 5/20/65	3-2

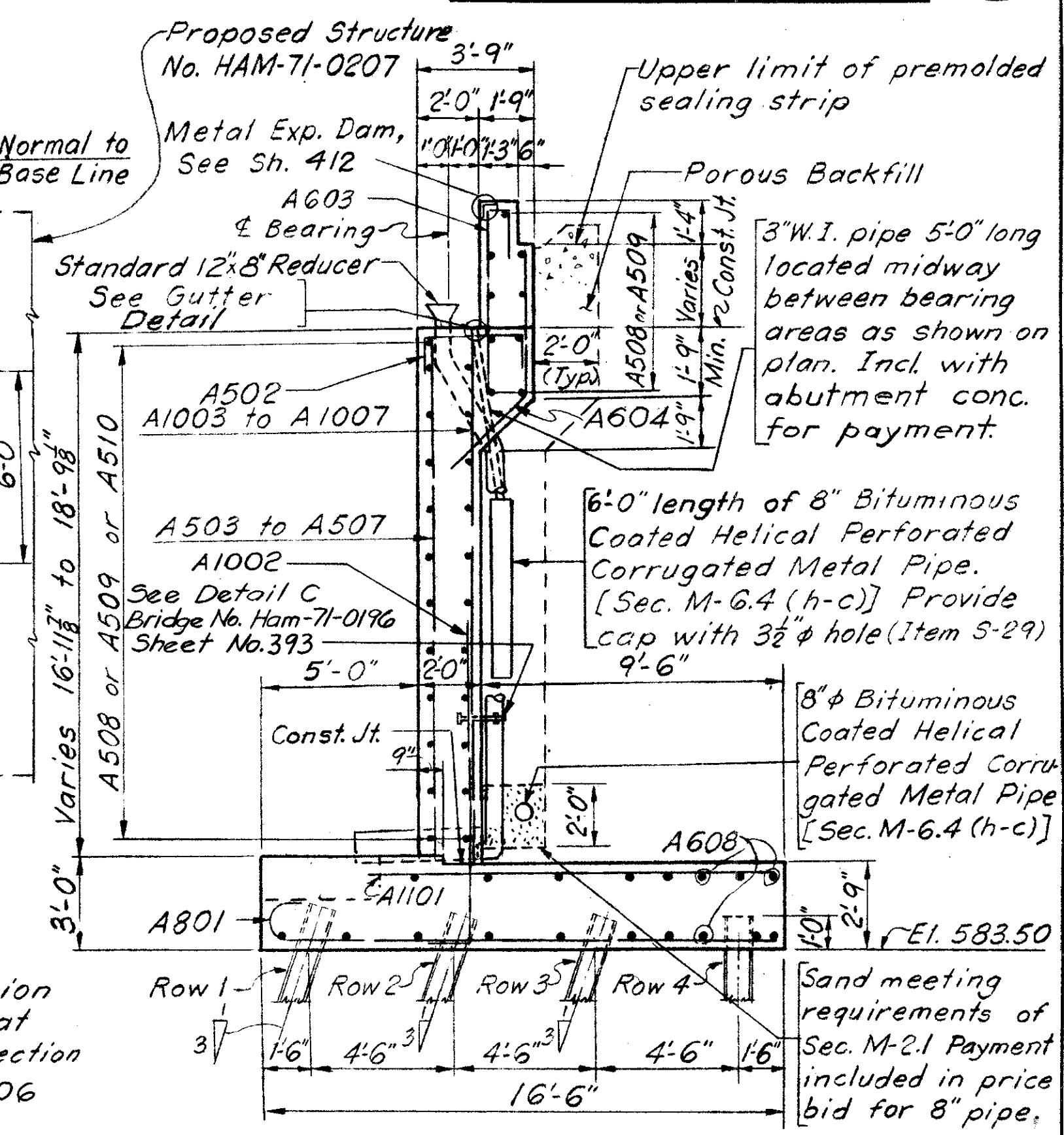
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		



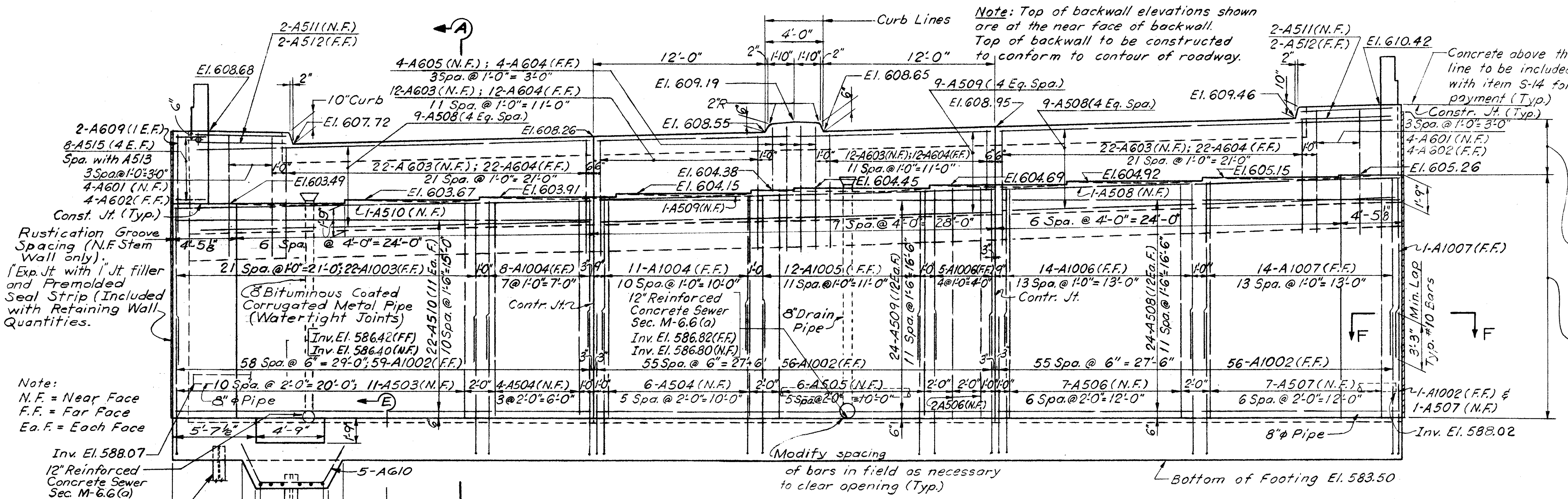
For lighting details see Sh. No. 216.
Note: The 2" Metal Conduit is included with Item 5-25 Metal Conduit, Sh. No. 220, for Payment.

ABUTMENT PLAN

Note: Superstructure drainage provision shown on Section A-A applies at fascia curb. For drainage connection at median curb see Sheet 406



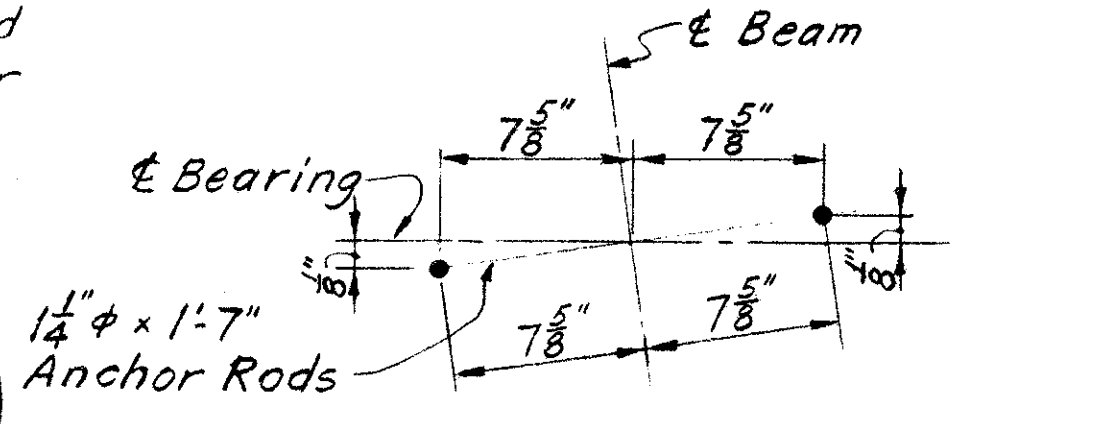
SECTION A-A



FRONT ELEVATION

Note:
N.F. = Near Face
F.F. = Far Face
Ea.F. = Each Face

Inv. El. 588.07
12" Reinforced Concrete Sewer Sec. M-6.6(a)
For Pile Layout see Footing Plan



DETAIL A
(Typical all beams)

1" Exp. Jt with 1" Jt filler and Type B Waterproofing (Incl. with Quantities for Abutment of Structure No. HAM-71-0203) (See Sect. B-B Bridge No. HAM-71-0207)
1" Exp. Jt with 1" Jt filler and Premolded Sealing Strip (Incl. with Quantities for Abutment of Structure No. HAM-71-0203)

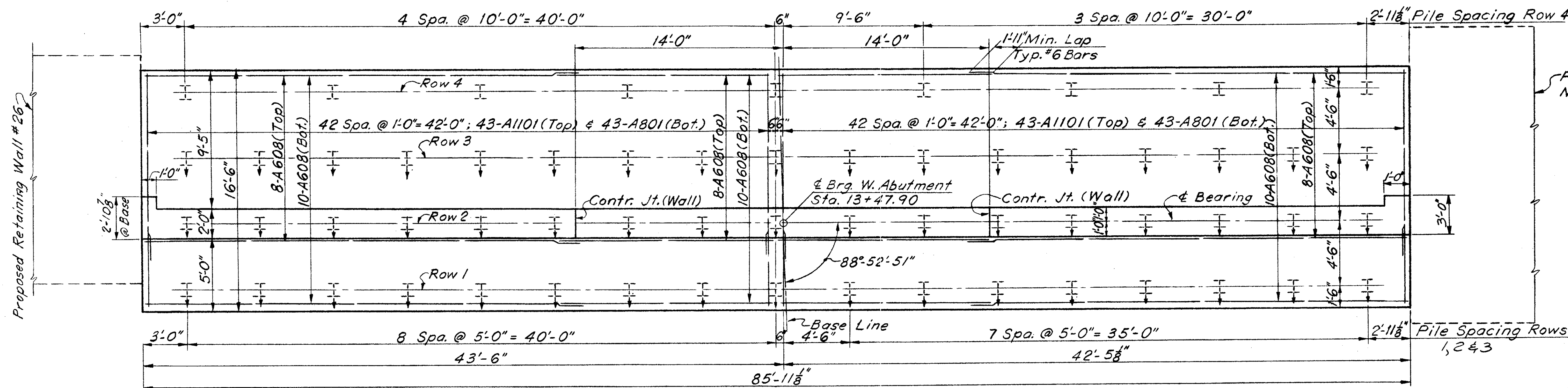
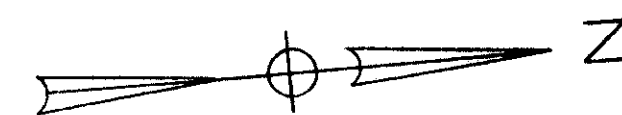
Work this sheet with sheet 406

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CINCINNATI, OHIO

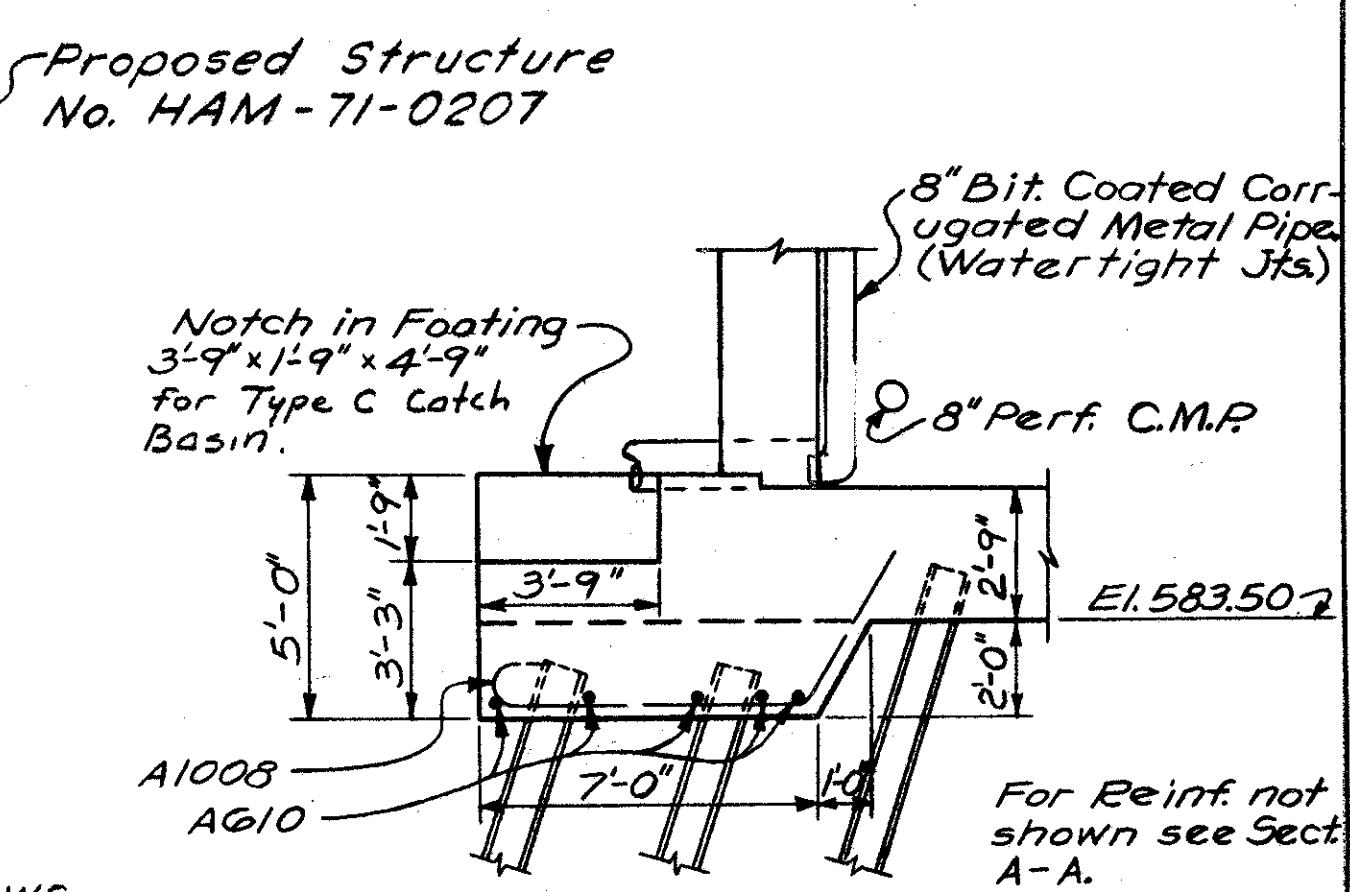
WEST ABUTMENT
BRIDGE NO. HAM-71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISOR
TFL	TFL		W.J.Z. 3-19-65	J.H.O. 5/20/65	

HAMILTON COUNTY
HAM-71-(1.66) (2.51)



FOOTING PLAN

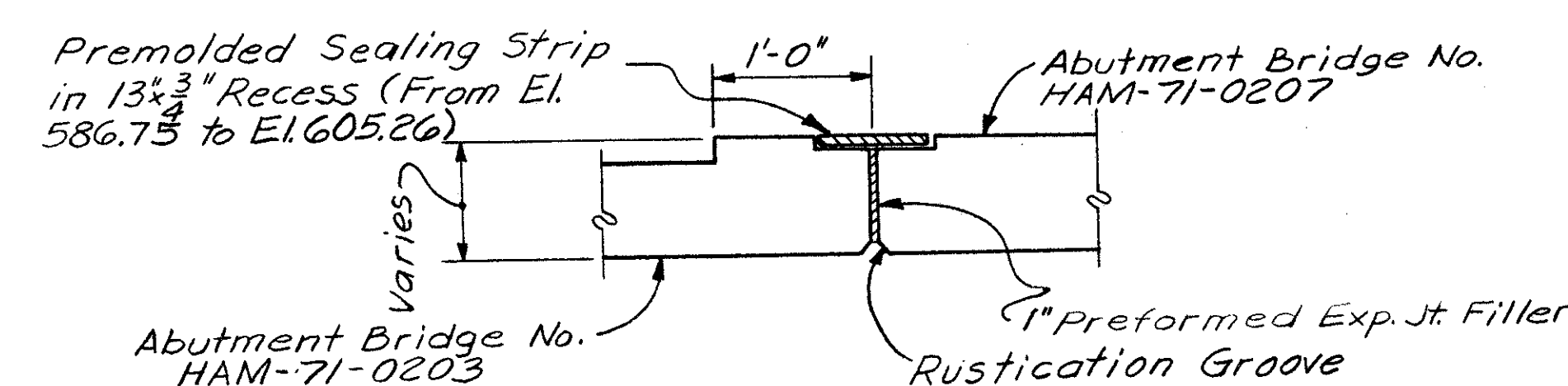


SECTION E-E

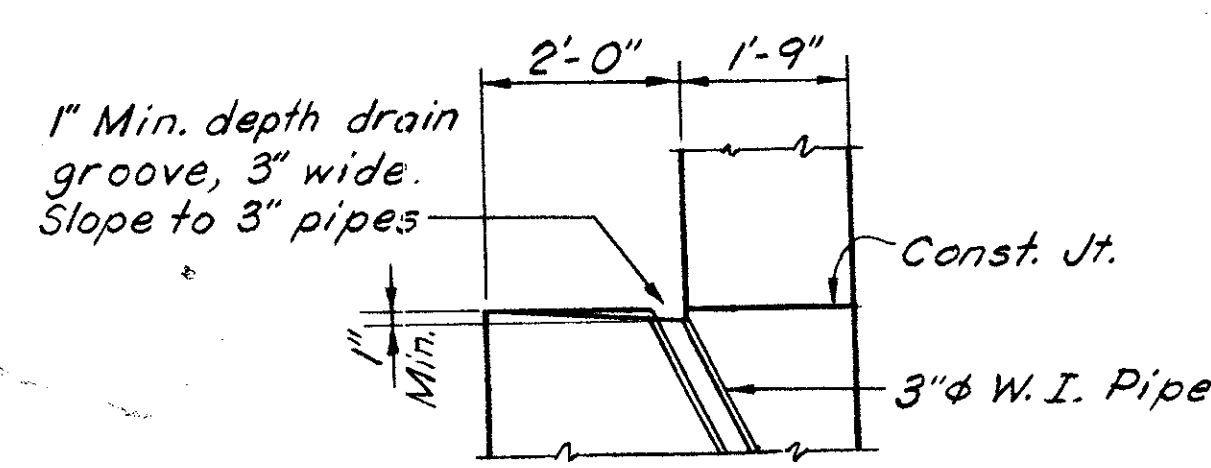
Notes:
Provide 3" clearance to reinforcing steel in footing.
All piles shall be 10BP42 Steel H piles.
Abutment concrete shall be Class "E"
Concrete above bridge seat construction joint shall not be placed until Superstructure Steel is in place.
Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.

PROCEDURE: Before the backwall is constructed the embankment behind the West abutment shall be placed and compacted up to the level of the existing ground (with a 1:1 slope from the bridge seat to the existing ground) and extending to the limits for excavation of unsuitable material.

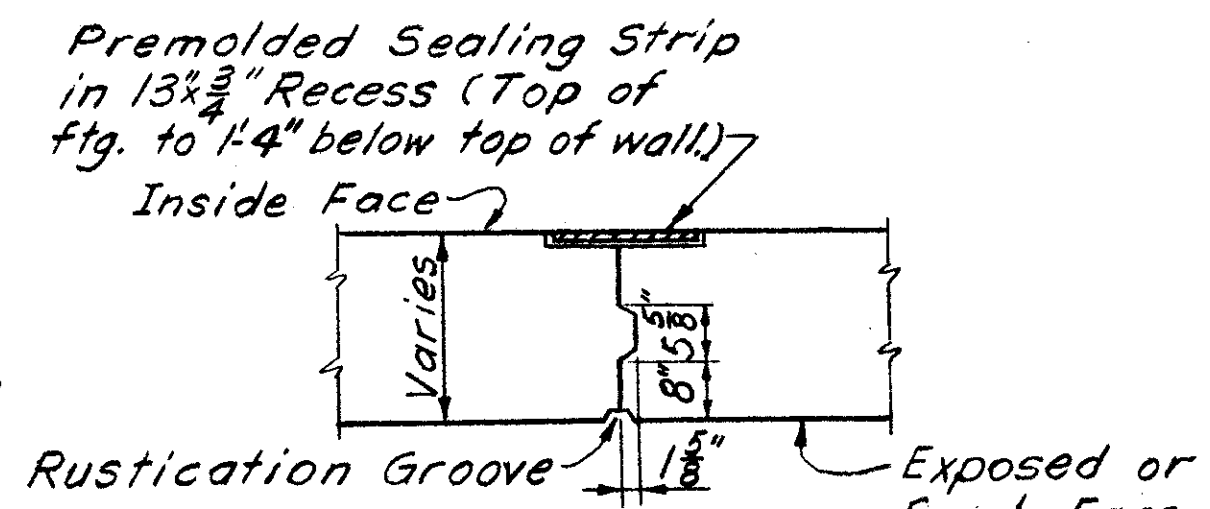
Note:
Porous Backfill 2 ft. thick, full length of abutment and wings shall extend up to the underside of the approach slab or to the finished ground surface.



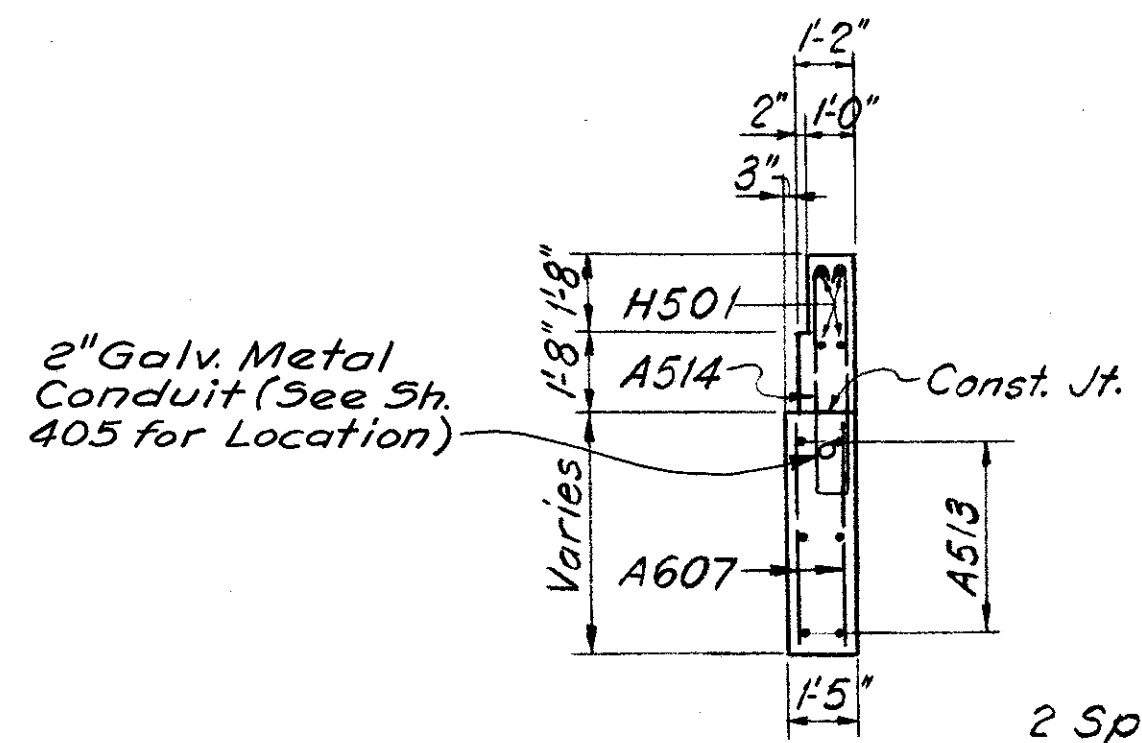
SECTION F-F



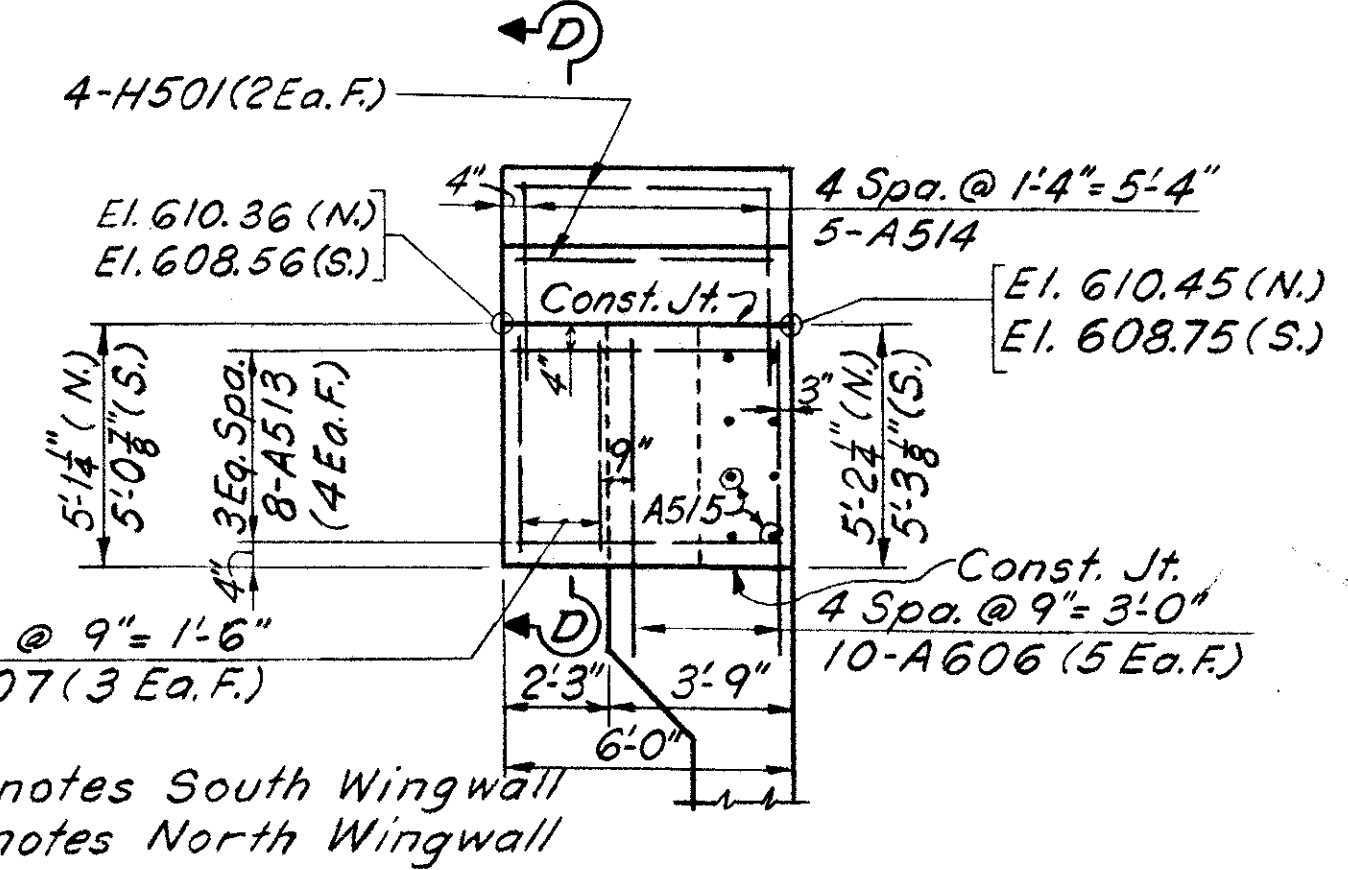
GUTTER DETAIL



CONTRACTION JOINT DETAIL

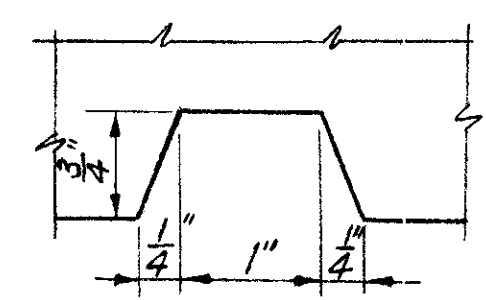


SECTION D-D

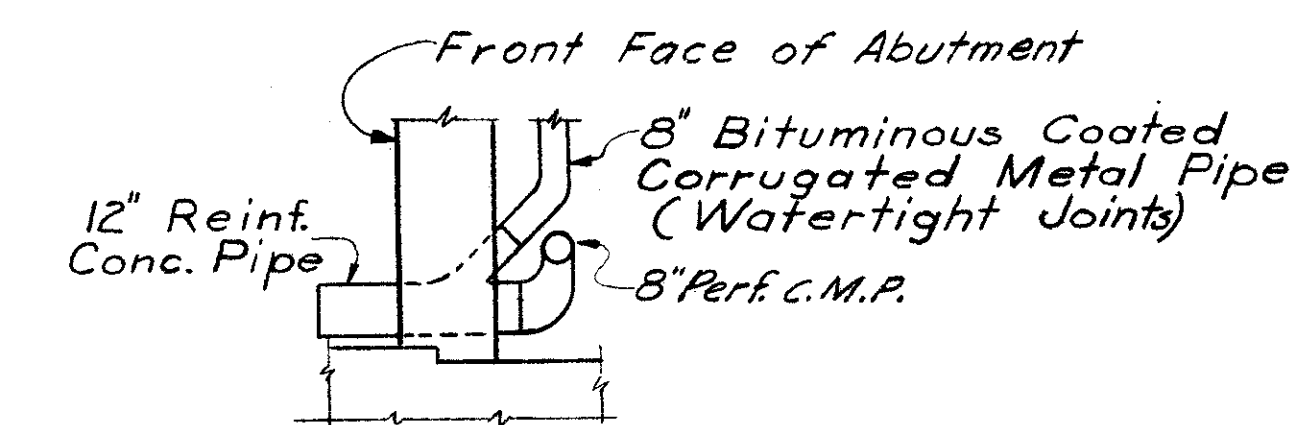


SECTION B-B

SECTION C-C (Similar, opposite hand except as noted)



RUSTICATION GROOVE DETAIL



At Median Curb of West Abut.

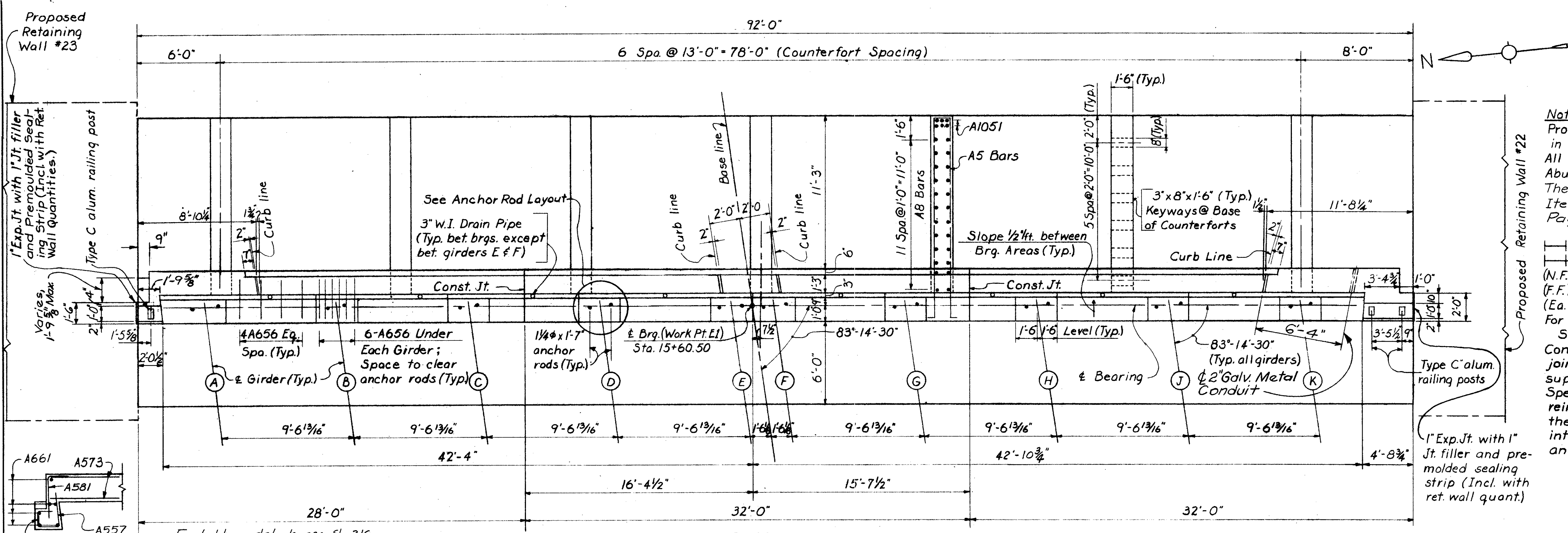
DRAINAGE CONNECTION DETAILS

Work this sheet with Sheet 405

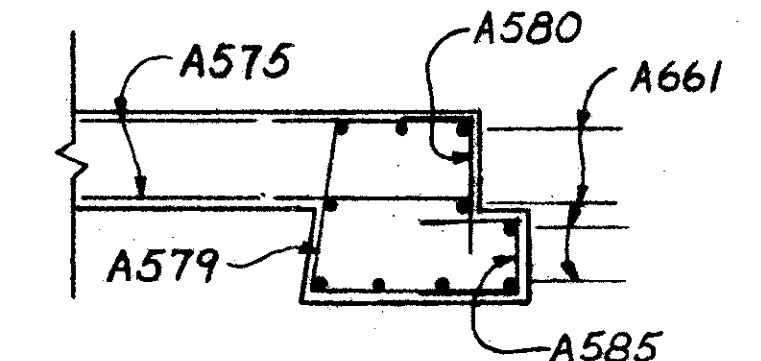
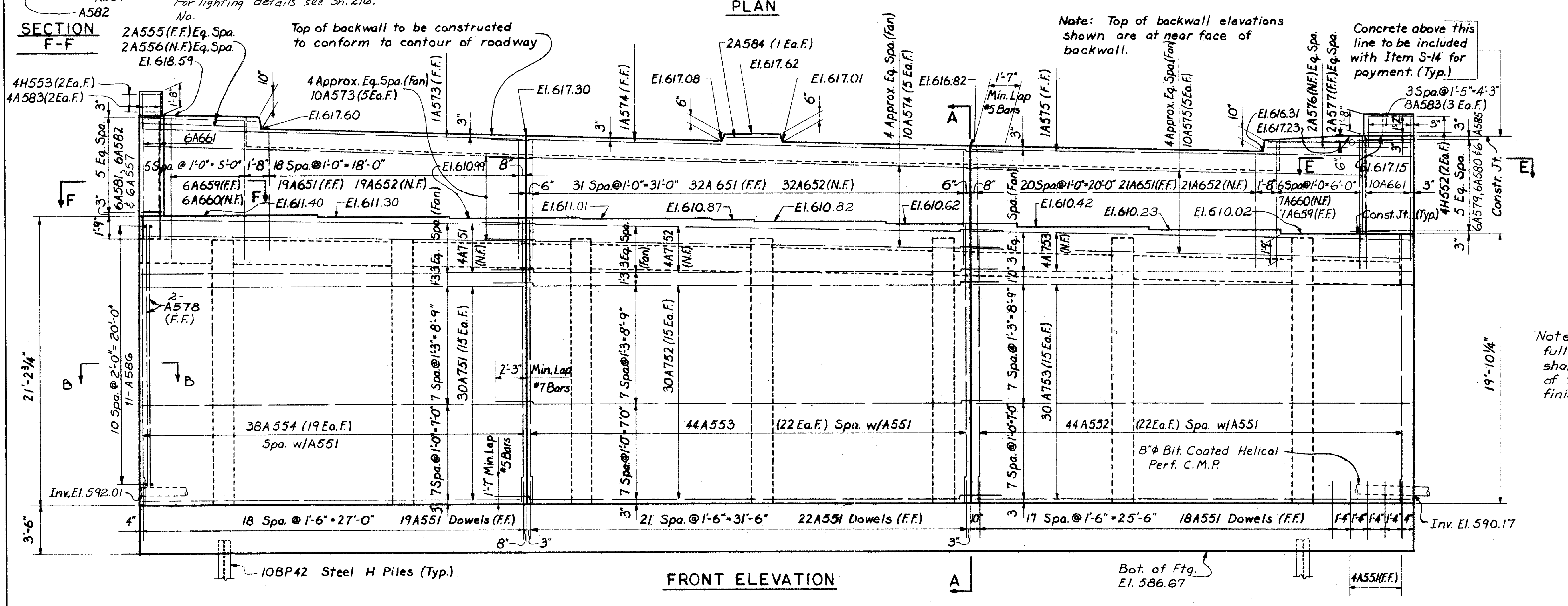
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
WEST ABUTMENT					
BRIDGE NO. HAM-71-0203					
DESIGNED TFL	DRAWN TFL	TRACED	CHECKED W.J.Z. 3-17-65	REVIEWED DATE JH 5/20/65	REVISED

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FEB 20 1985

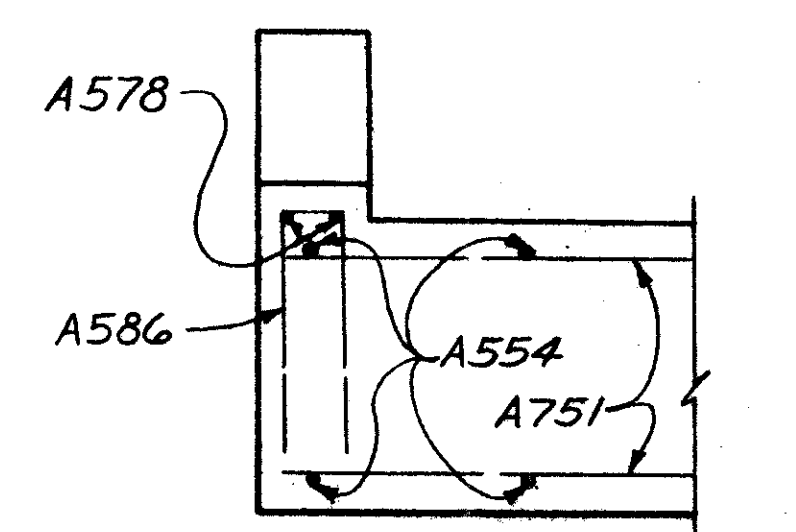
**HAMILTON COUNTY
HAM-71-(1.50) (2.51)**



Notes:
 Provide 3" clearance to reinforcing steel in footing and 2" clearance in the wall. All piles shall be 10BP42 Steel H Piles. Abutment concrete shall be Class 'E'. The 2" Metal Conduit is included with Item 5-25 Metal Conduit, Sh. 220, for Payment.
 | denotes vertical pile.
 / denotes batter pile (batter 1 on 3).
 (N.F.) denotes near face.
 (F.F.) denotes far face.
 (Ea.F.) denotes each face.
 For details of aluminum railing see Standard drawing AR-1-57.
 Concrete above bridge seat construction joint shall not be placed until superstructure steel is in place. Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.



SECTION E-E



SECTION B-B

Note: Porous backfill 2 ft thick, full length of abutment and wings, shall extend up to the underside of the approach slab or to the finished ground surface.

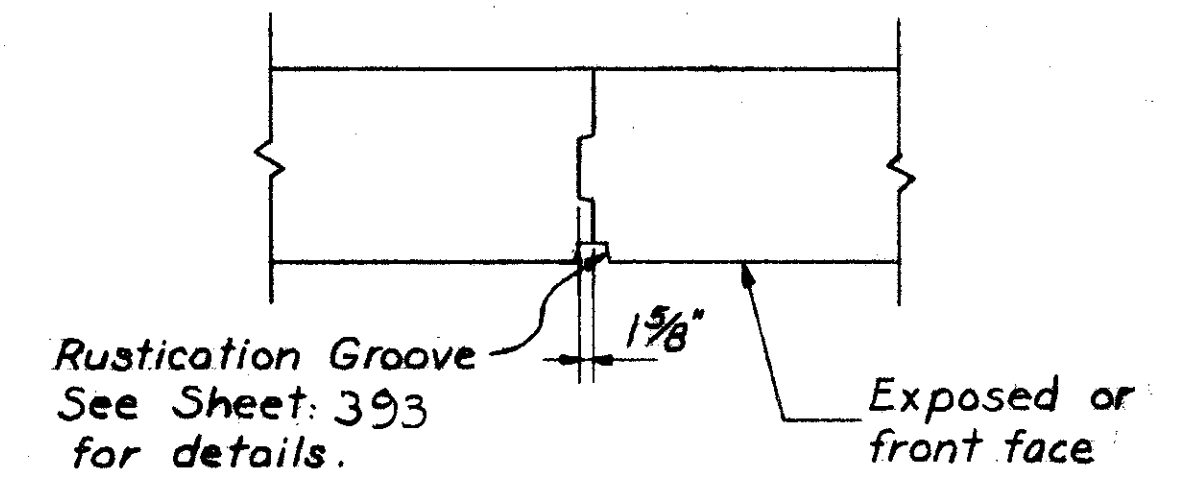
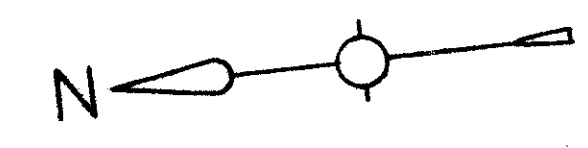
Work this sheet with Sheet 408

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 CINCINNATI, OHIO

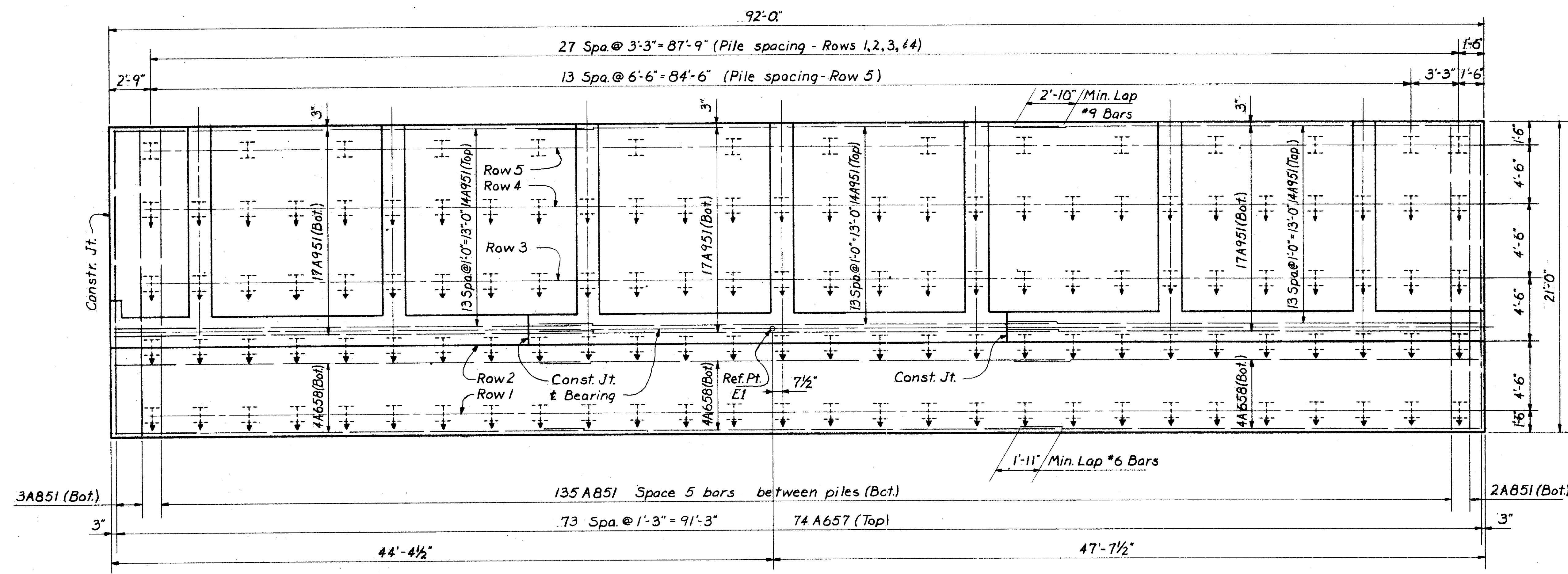
EAST ABUTMENT
 BRIDGE NO. HAM - 71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	AFS		W.J.Z.	J40	
			3-19-65	5/20/65	

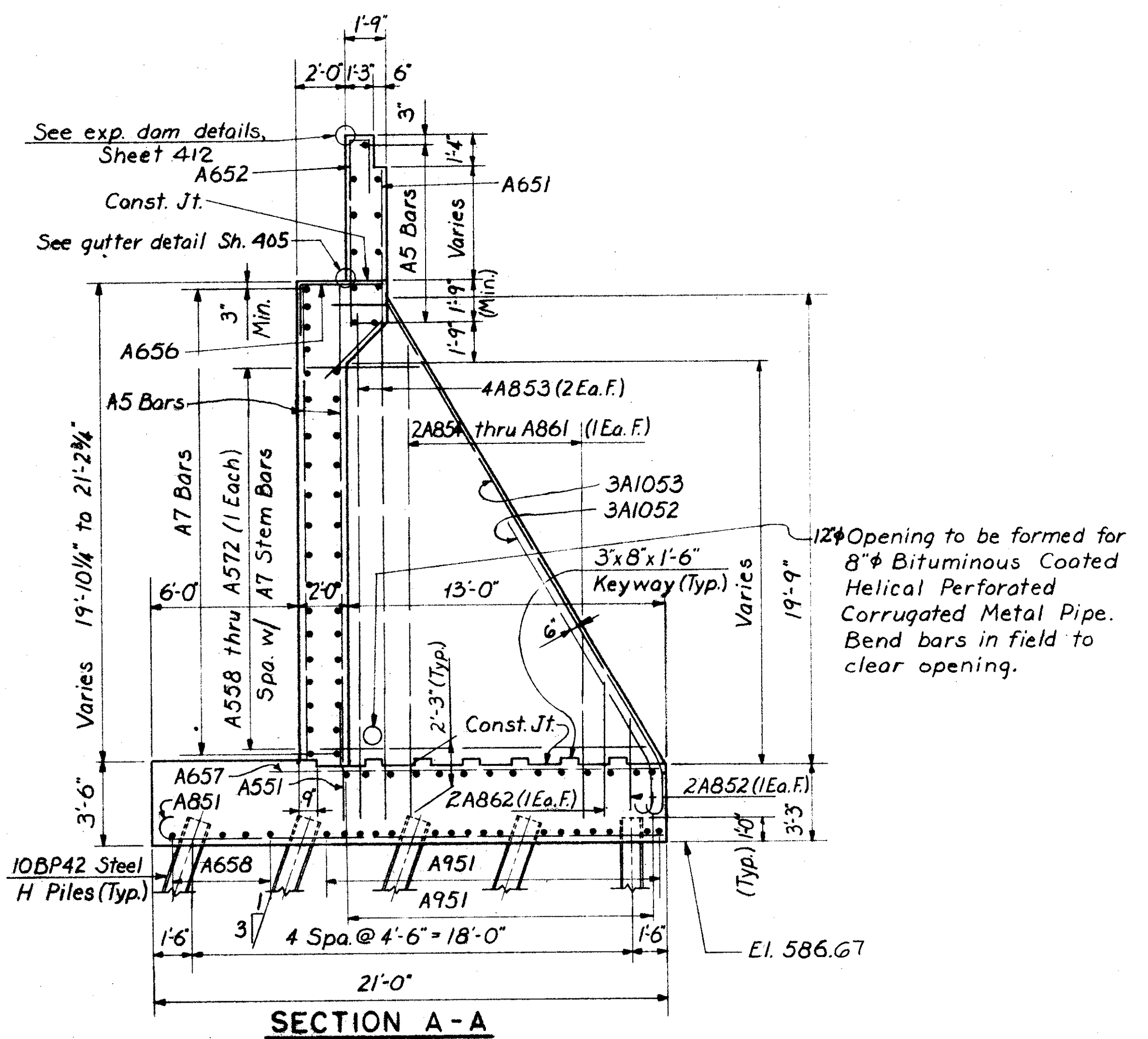
HAMILTON COUNTY
HAM-71-(1.58) (2.51)



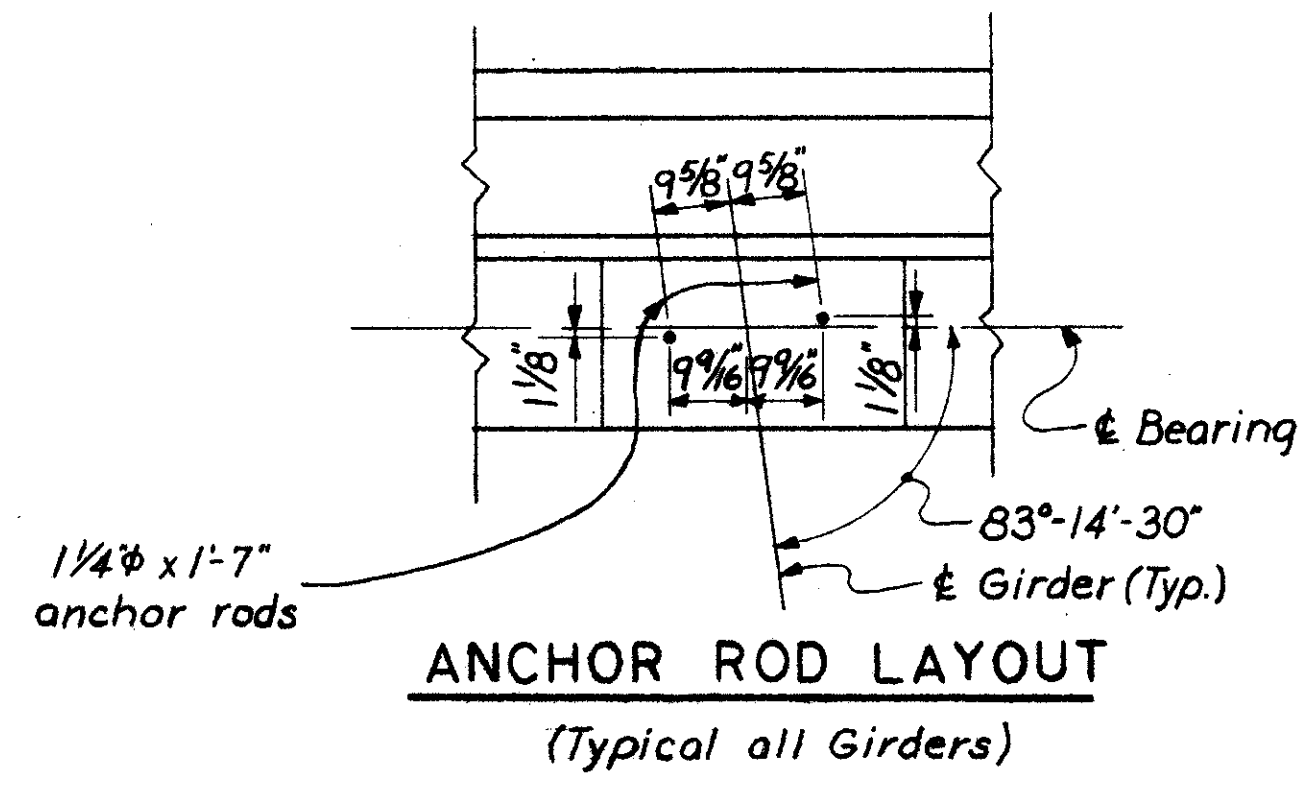
CONSTRUCTION JOINT DETAIL



FOOTING PLAN



SECTION A-A



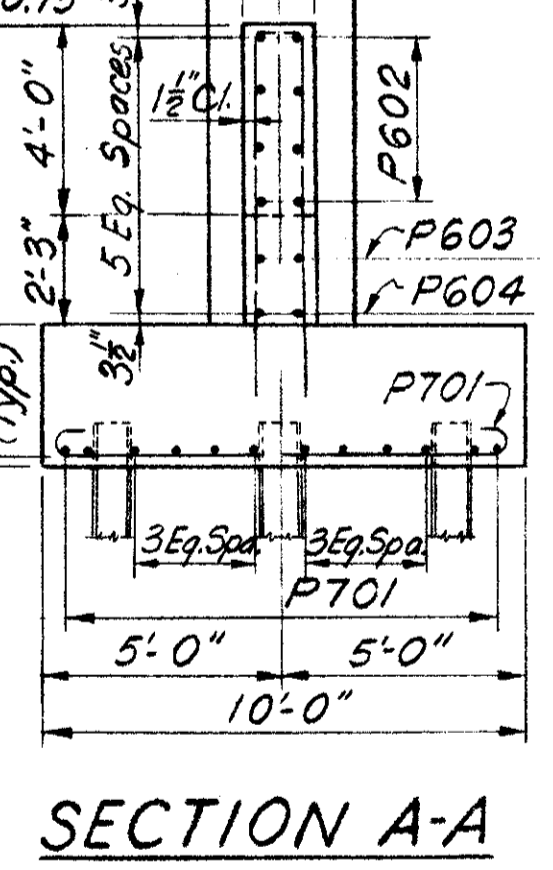
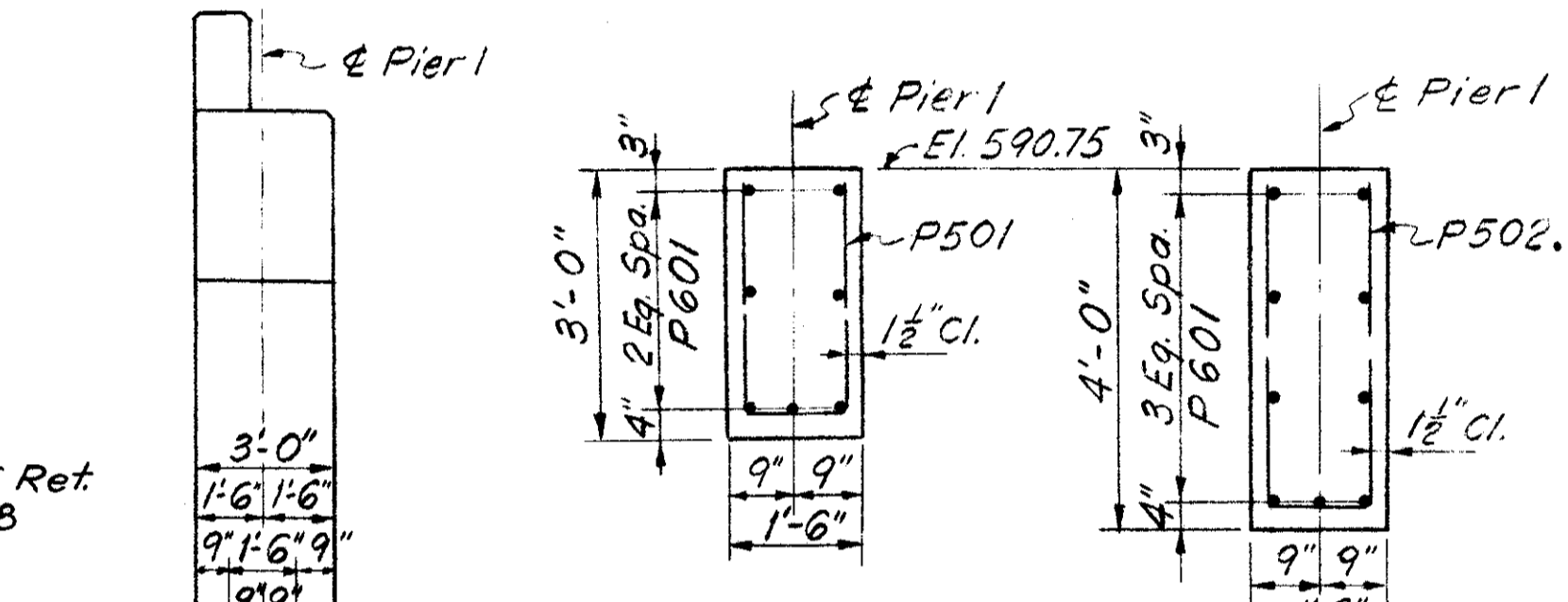
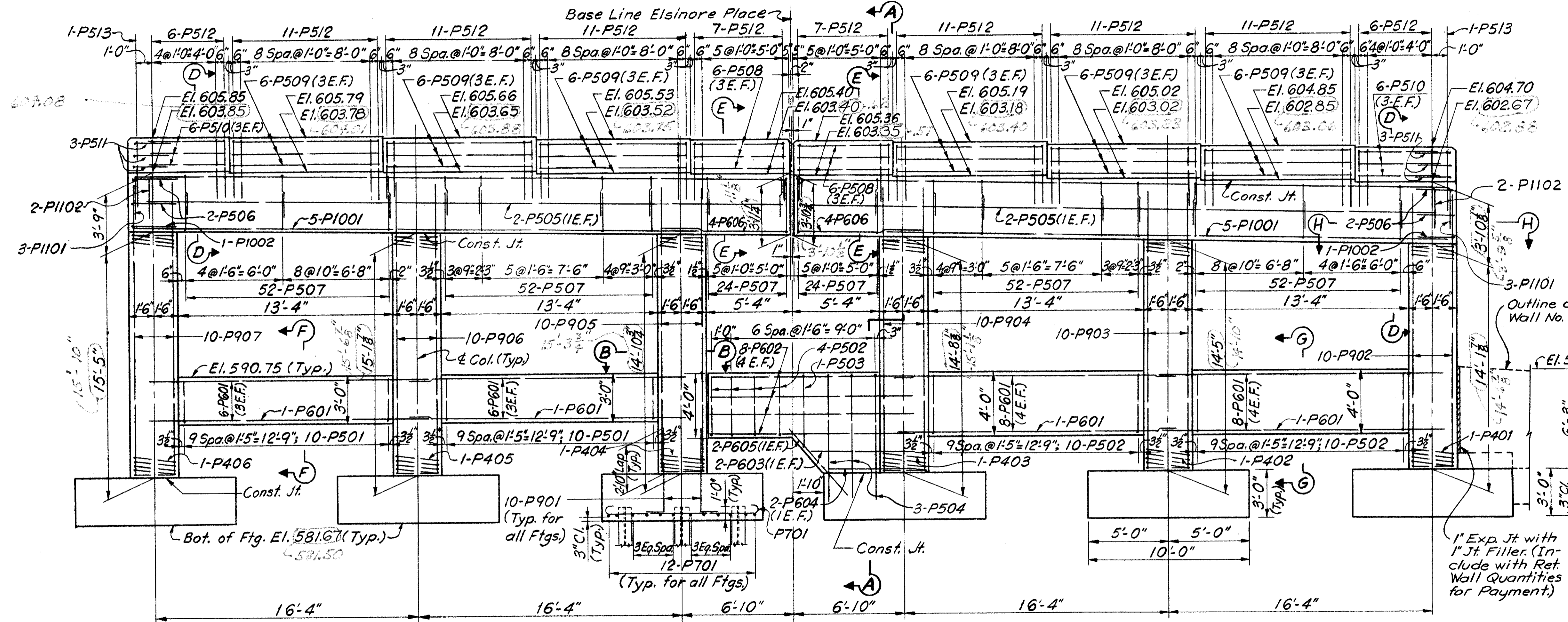
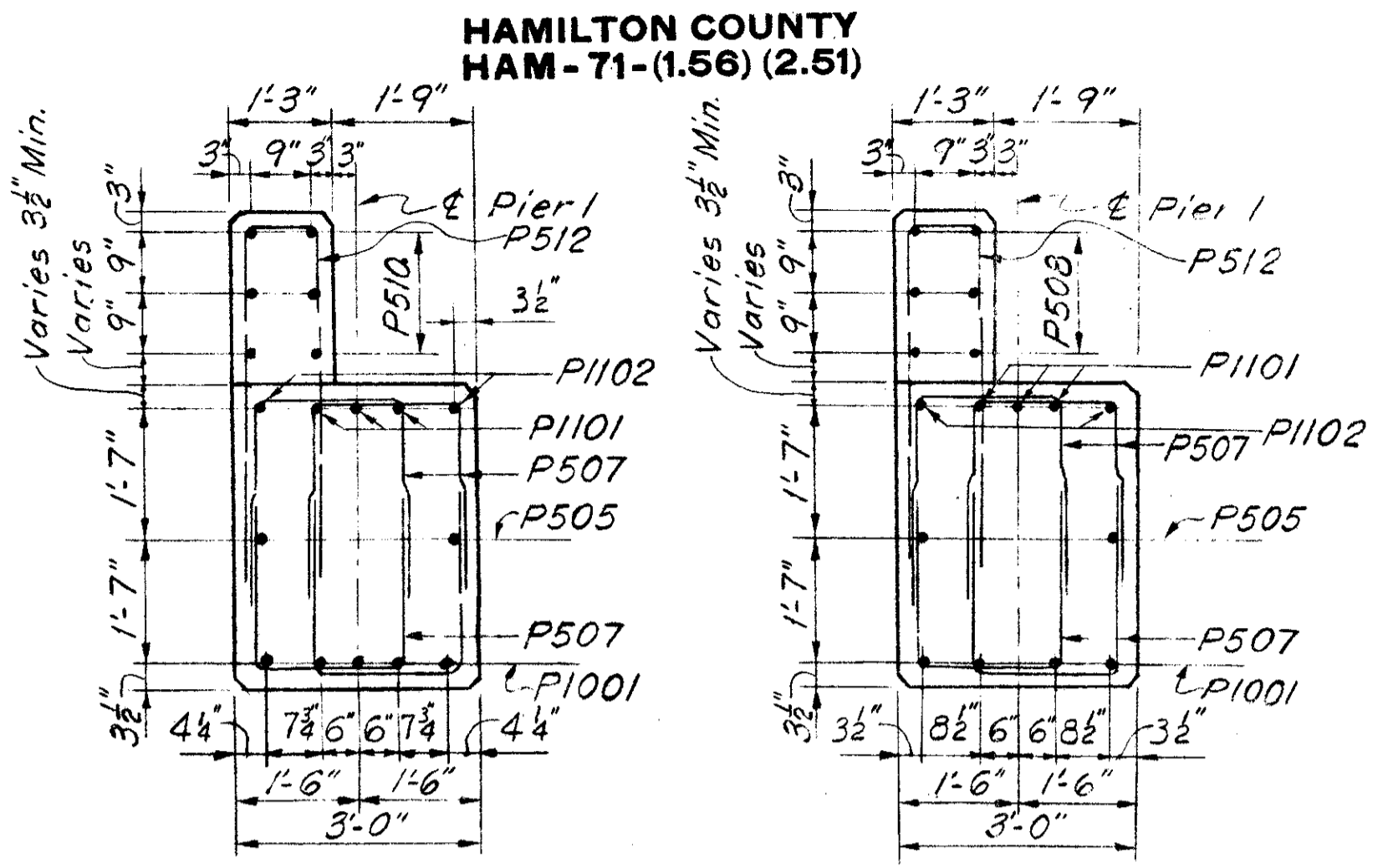
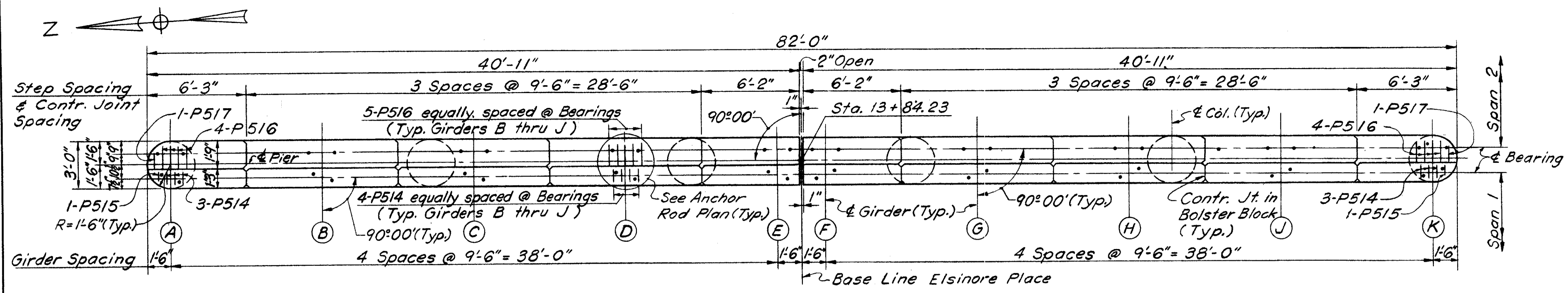
MICROFILMED
FEB 20 1965

Work this sheet with Sheet 407

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CINCINNATI, OHIO

EAST ABUTMENT
BRIDGE NO. HAM-71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	AFS		W.J.Z. 3-1965	JH 5/20/65	



Notes

Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.

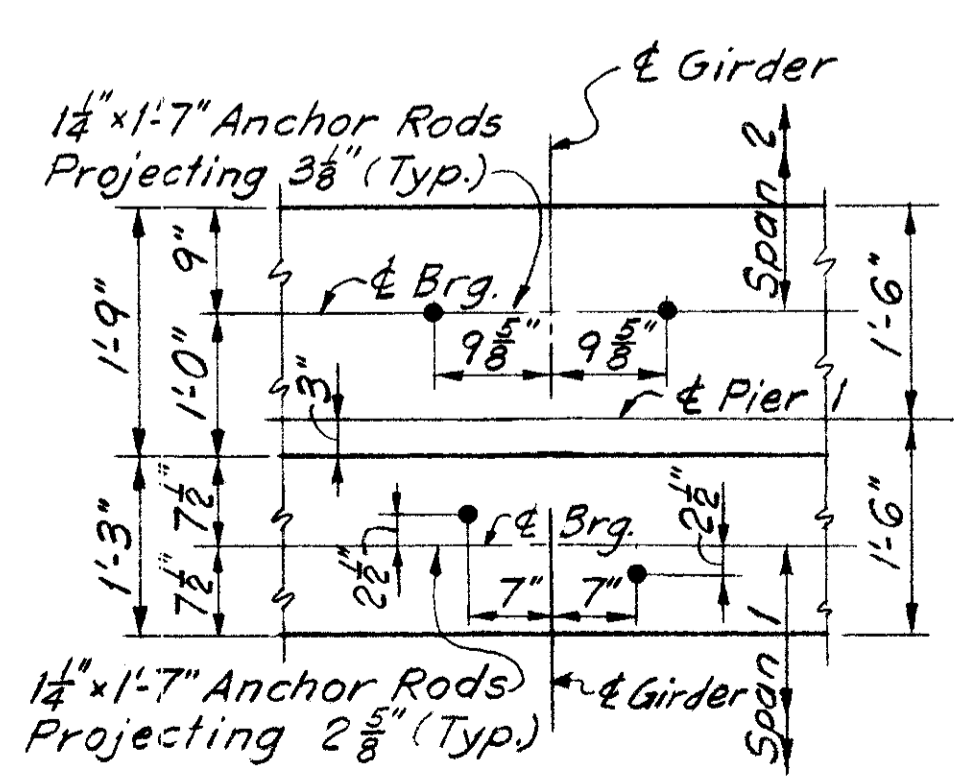
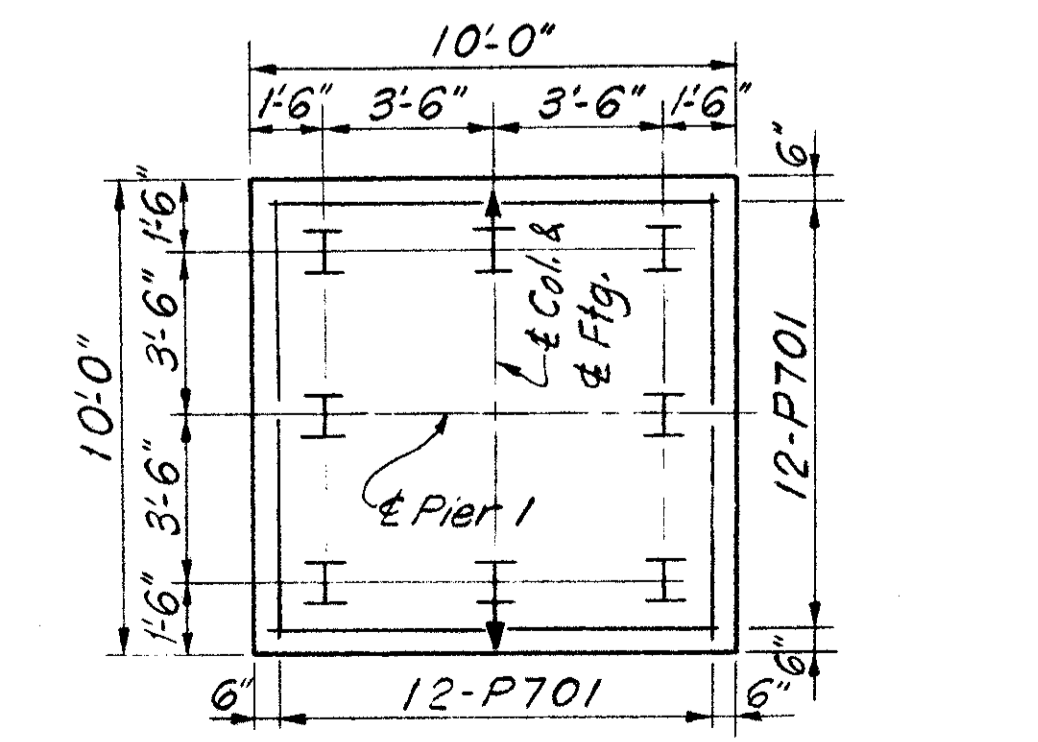
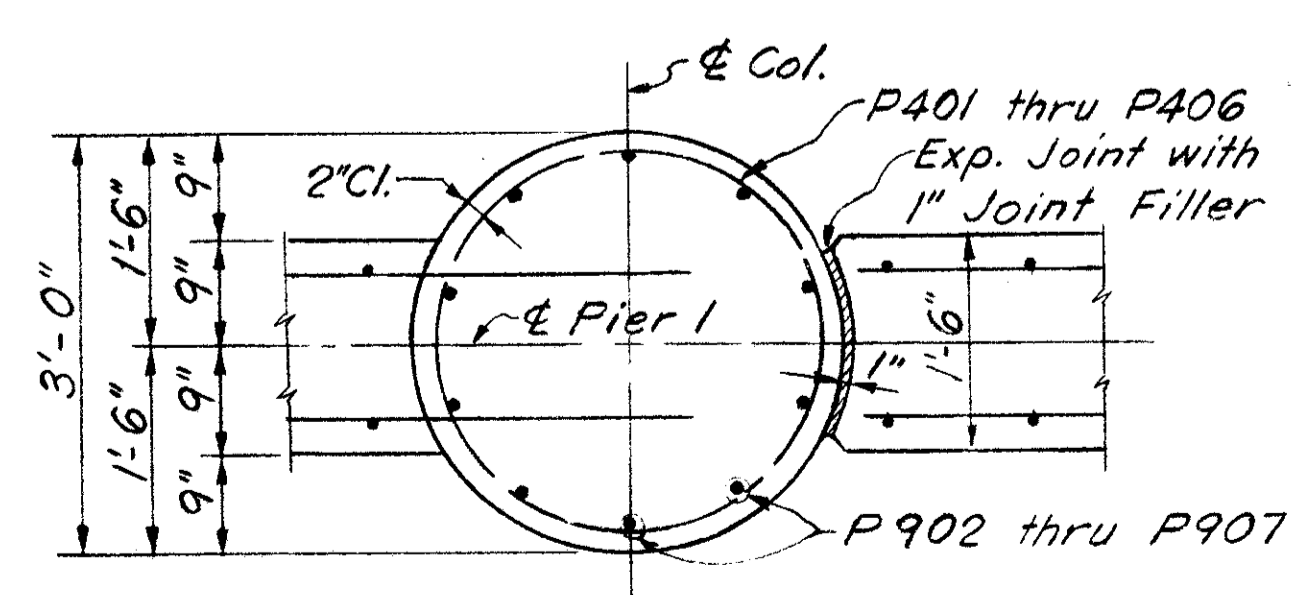
All piles shall be 10BP42 Steel H-Piles.

Concrete in footing shall be Class "E" and concrete in columns and caps shall be Class "C."

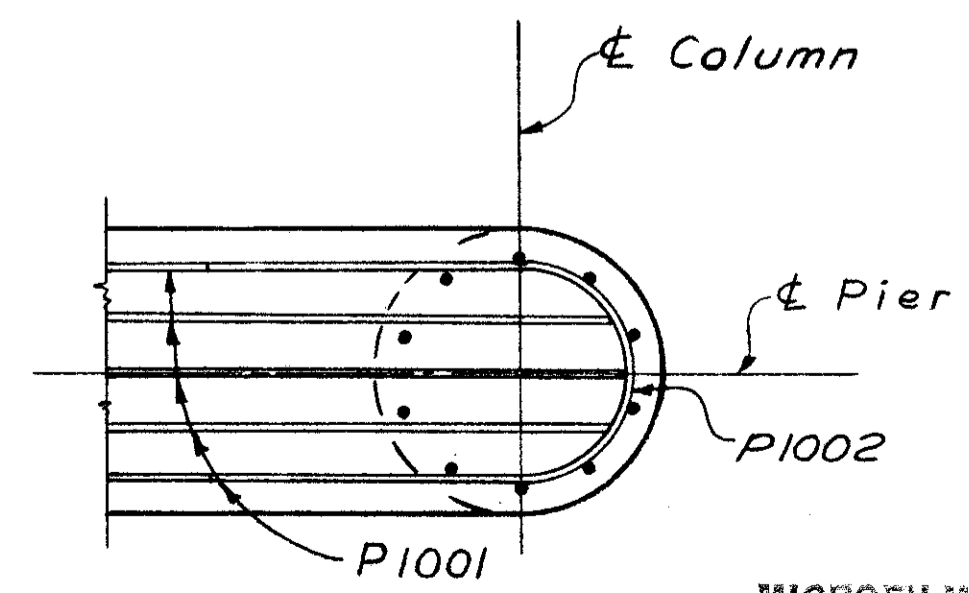
Concrete in walls between columns shall be Class "C" and shall be included with Pier 1 Quantities for payment.

E.F. denotes Each Face

← denotes batter pile 1:4



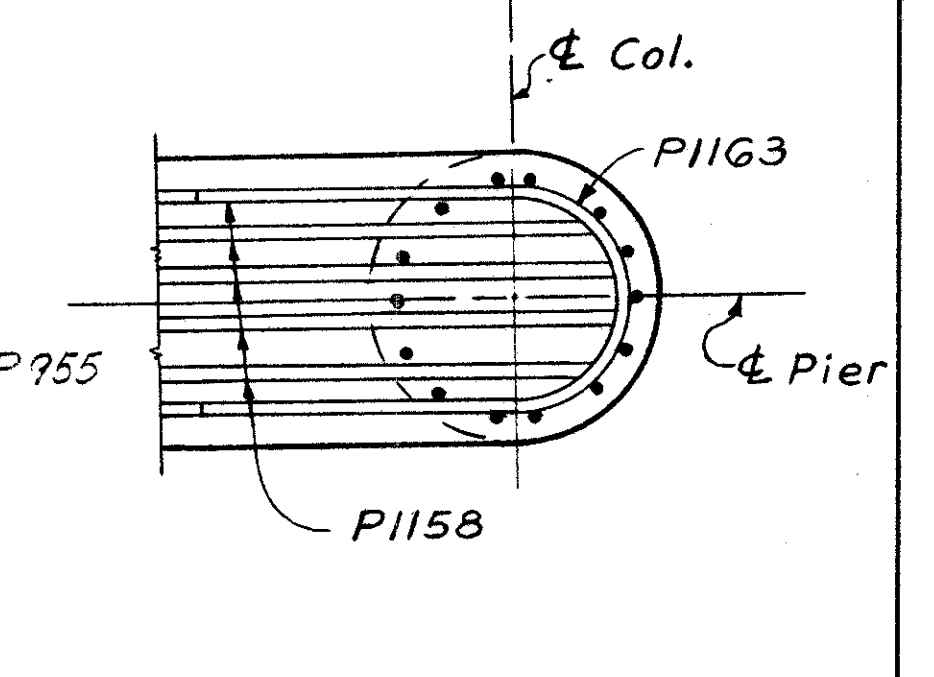
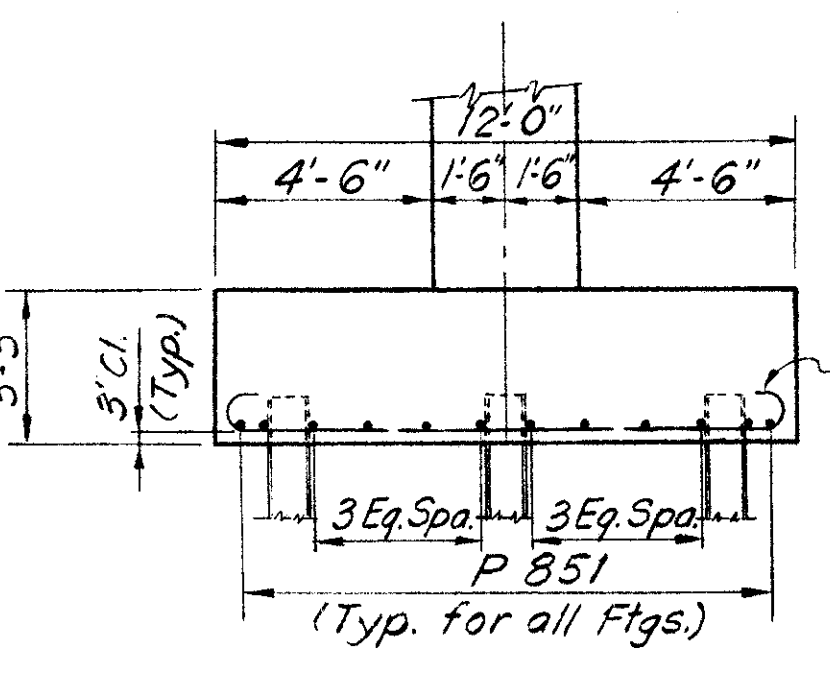
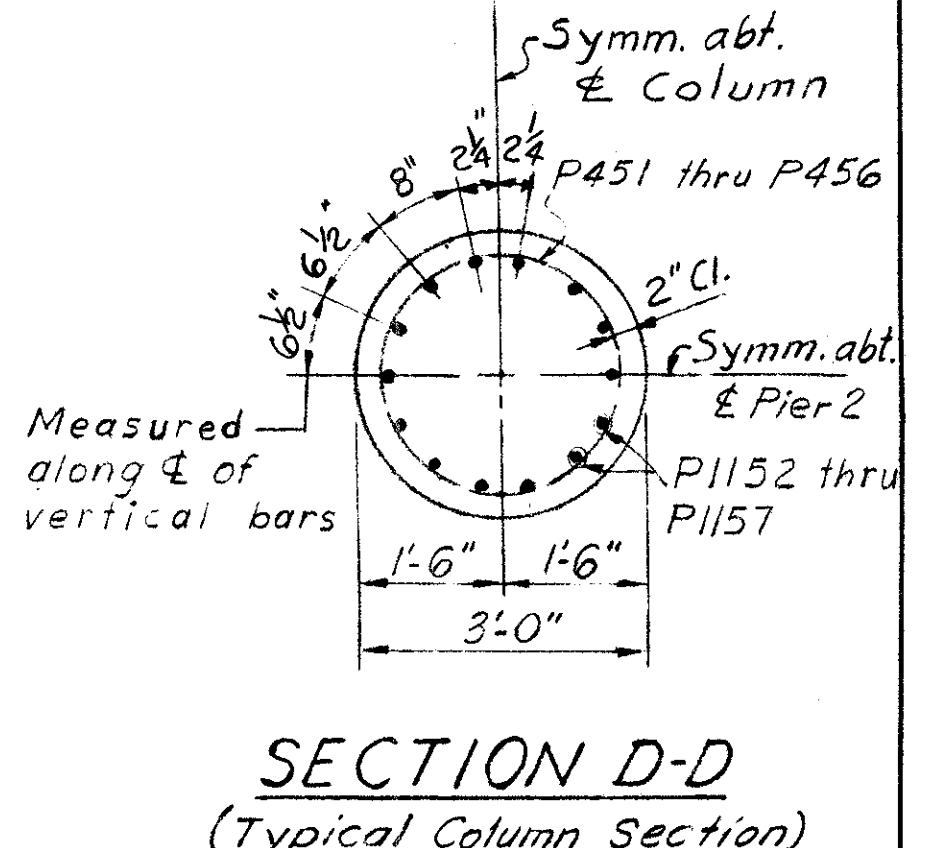
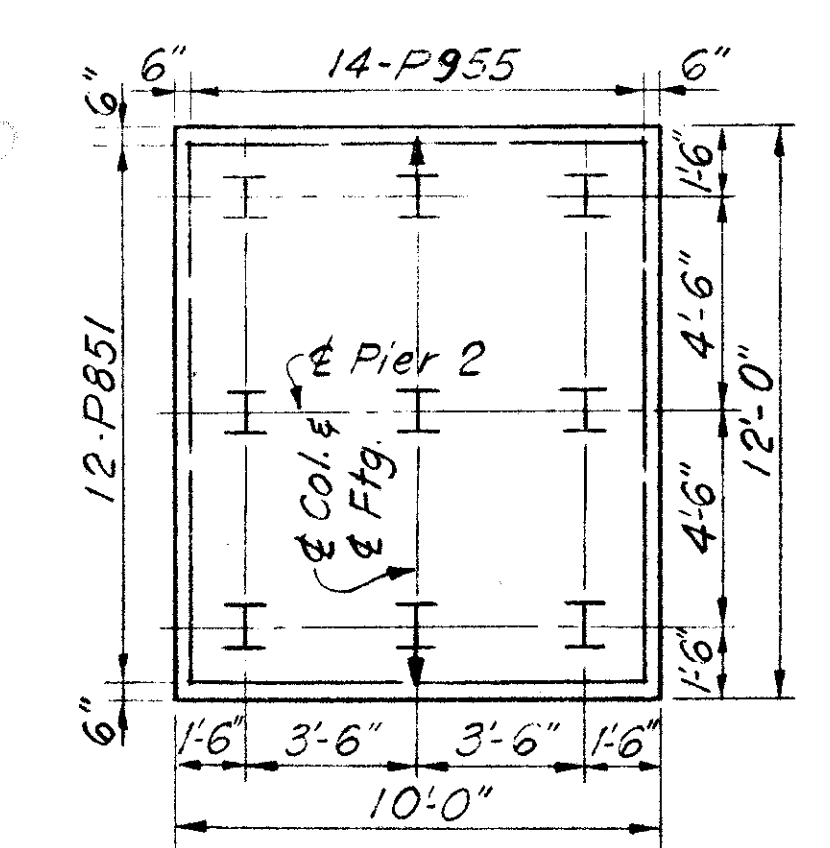
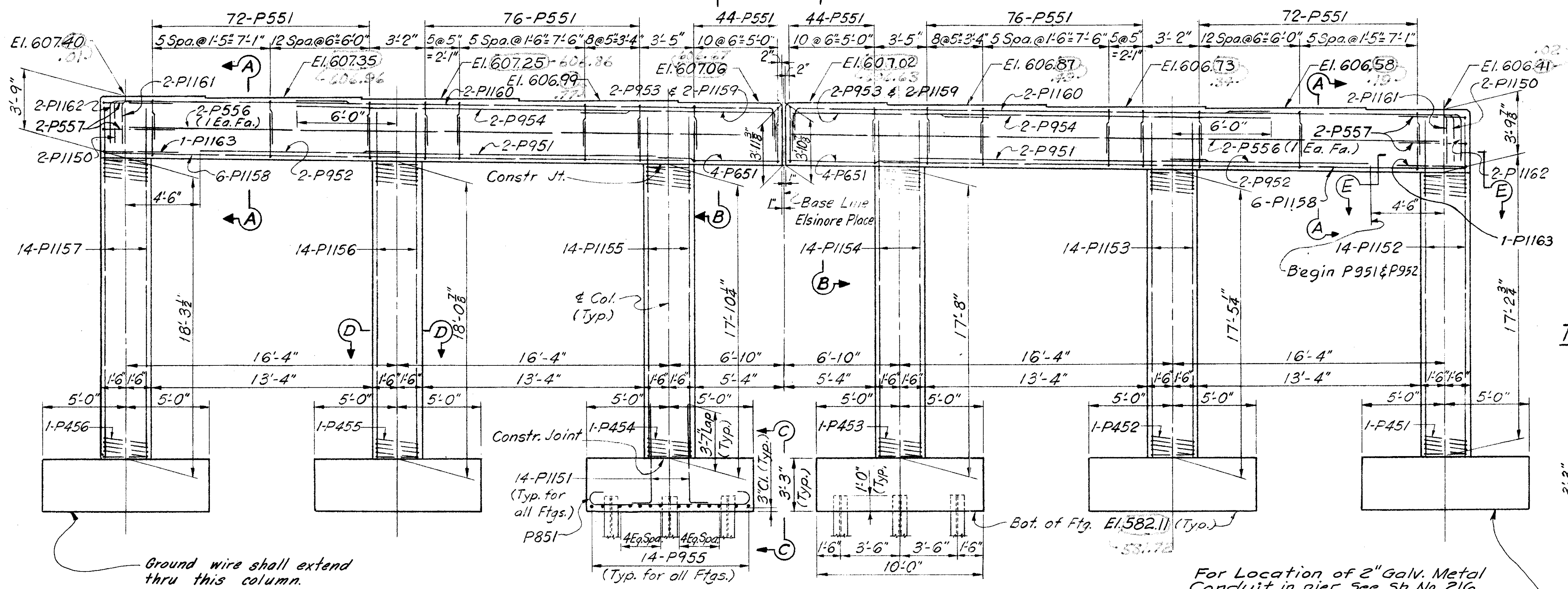
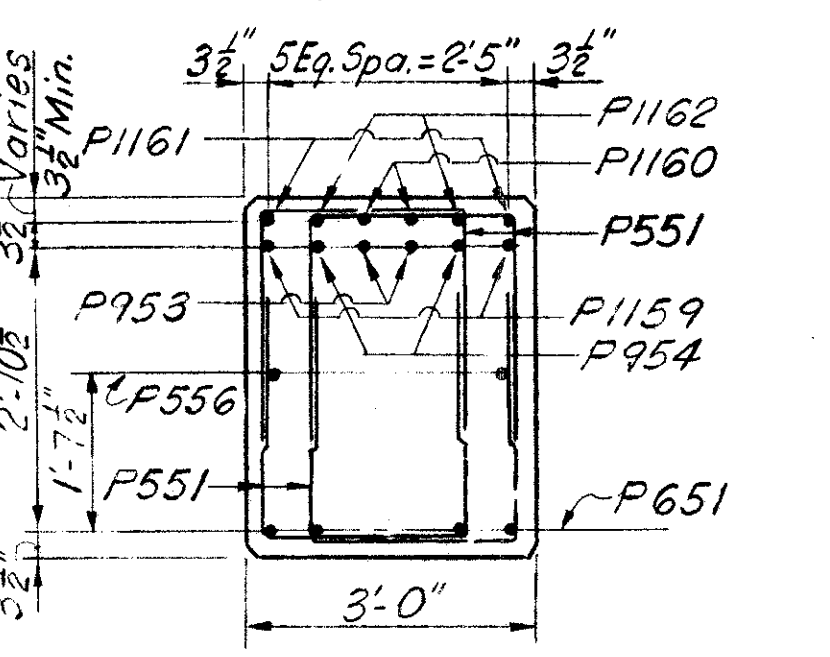
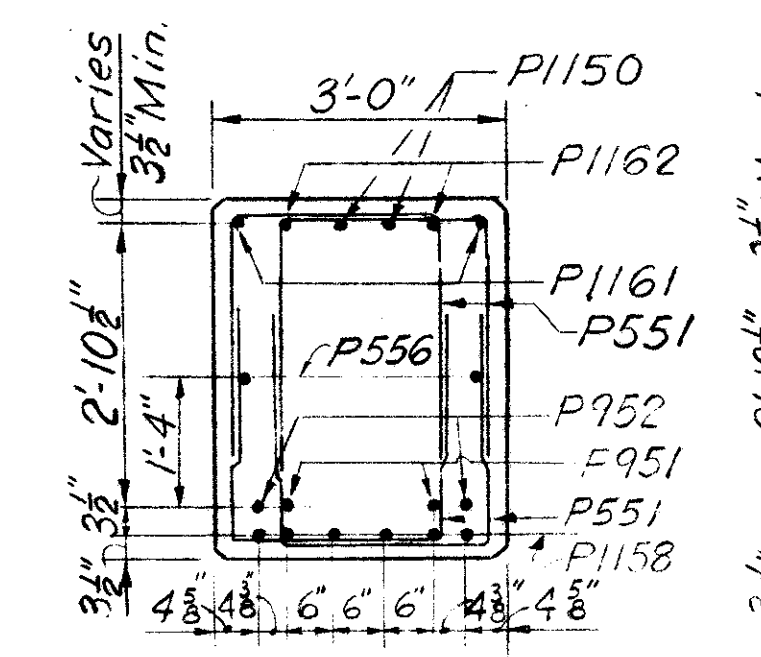
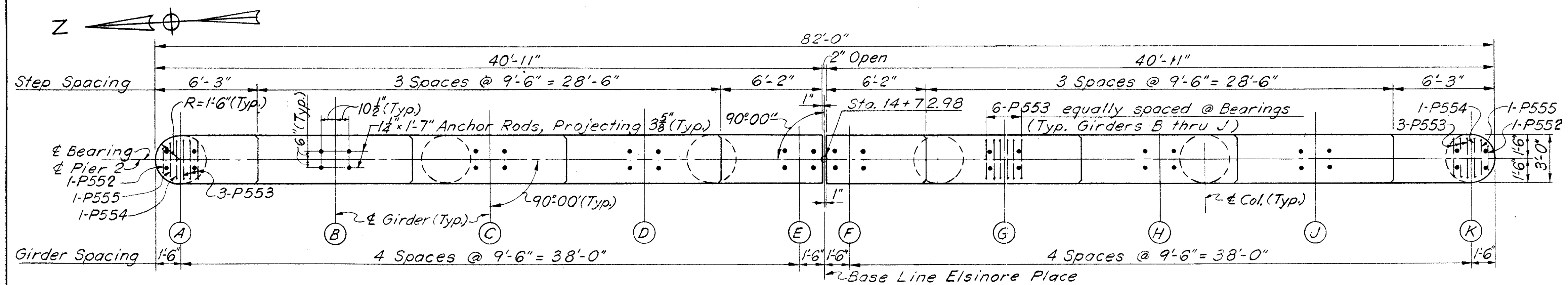
For Location of 2" Galv. Metal Conduit See Sh. No. 216.



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HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
PIER 1				
BRIDGE NO. HAM - 71-0203				
DESIGNED JYH	DRAWN TFL	TRACED	CHECKED D.W.	REVIEWED DATE JHO 5/20/65

**HAMILTON COUNTY
HAM-71-(156) (2.51)**



Notes:
 Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of anchor rod holes.
 Concrete in footing shall be Class "E" and concrete in columns and caps shall be Class "C".
 All piles shall be 10BP42 Steel H-Piles.
 E.F. denotes Each Face.
 1:4 denotes batter pile 1:4

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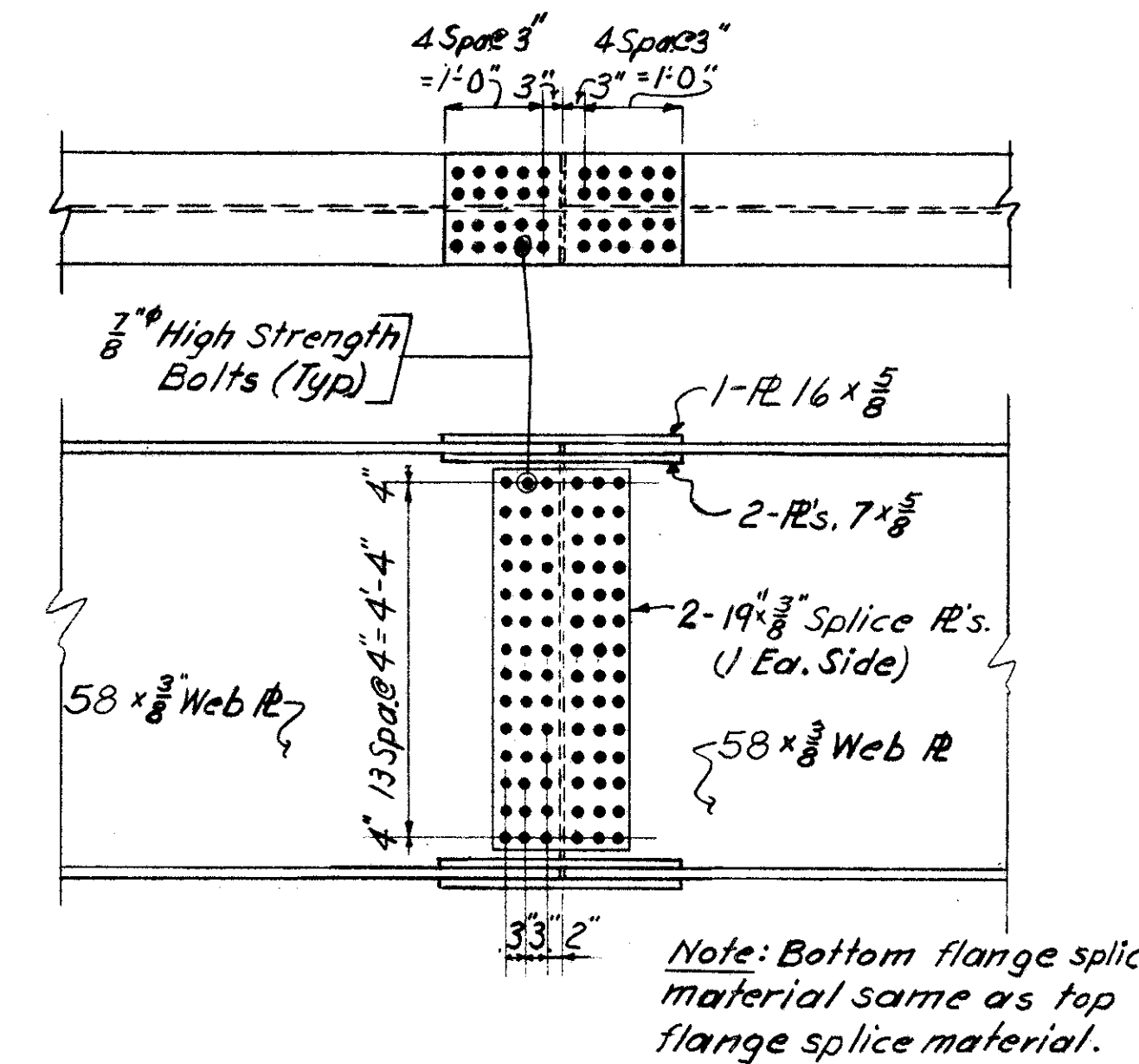
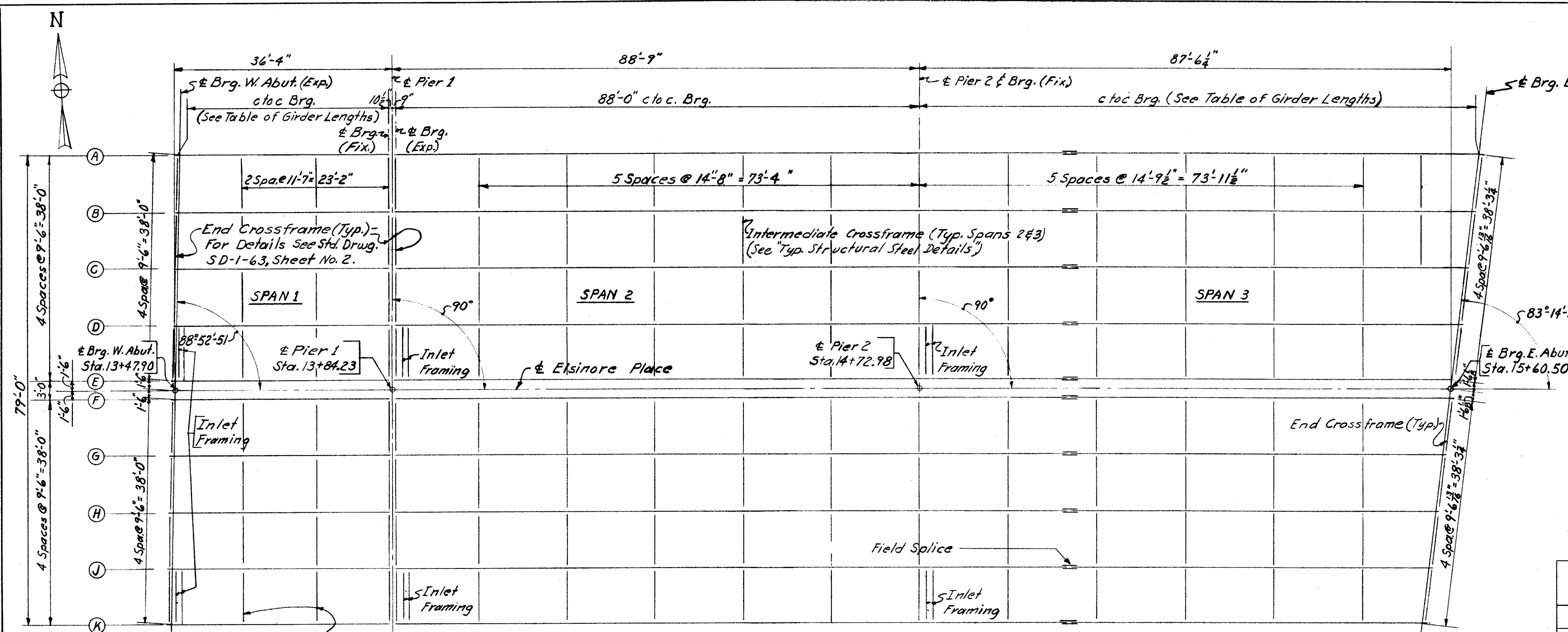
PIER 2

BRIDGE NO. HAM - 71-0203

DESIGNED JYH	DRAWN TFL	TRACED	CHECKED D.W.	REVIEWED DATE JH 5/10/65	REVISED
			3-19-65		

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HAMILTON COUNTY
HAM-71-(156) (2.51)

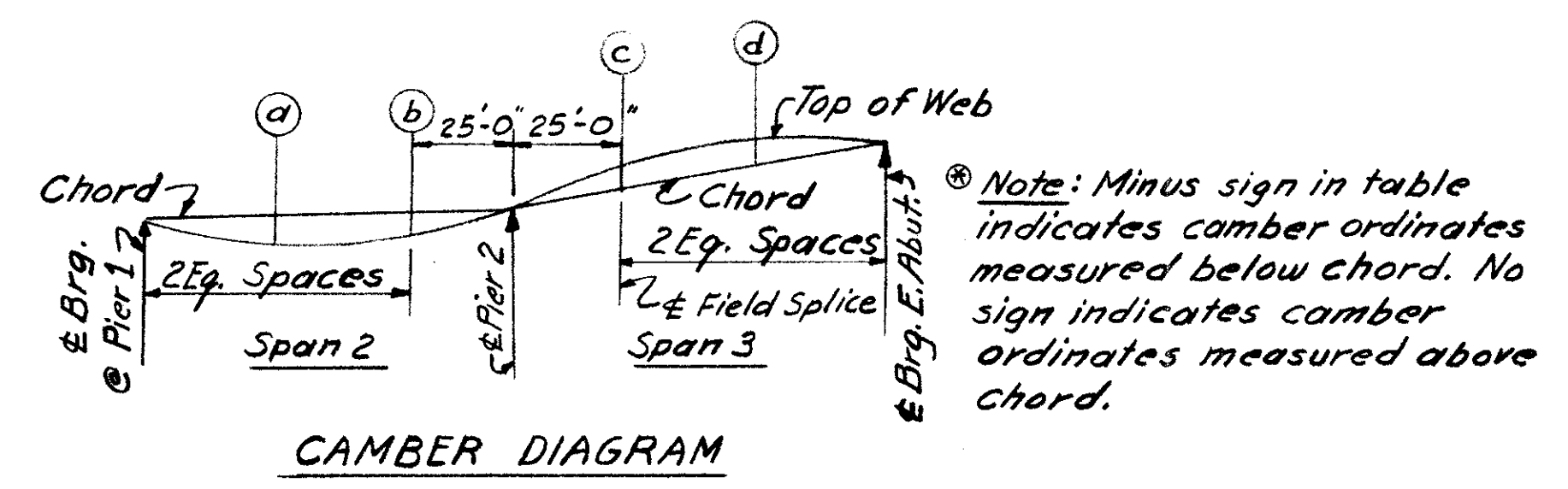
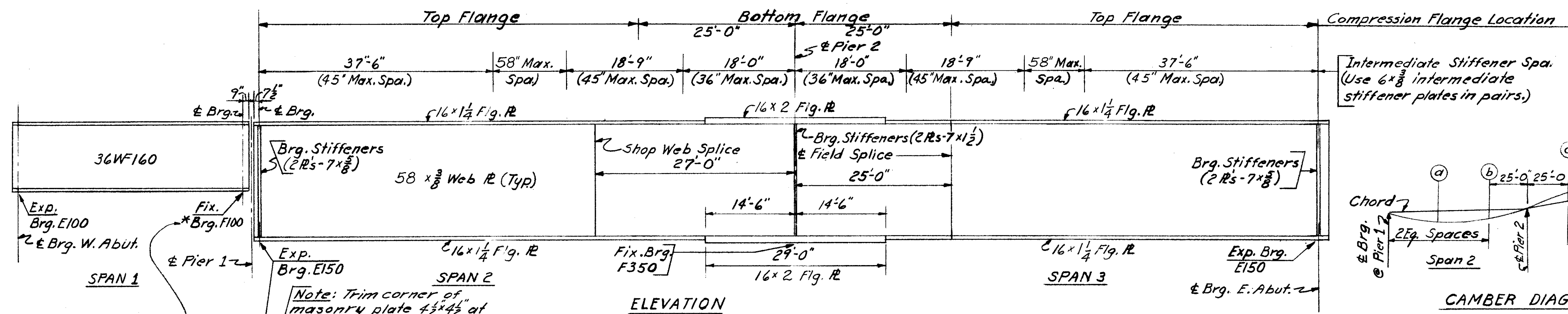


DEFLECTION AND CONVEXITY TABLE
SPAN 1 @ E OF SPAN

GIRDER	A	B	C	D	E	F	G	H	J	K
Deflection due to Weight of Steel	0	0	0	0	0	0	0	0	0	0
Deflection due to Remaining D.L.	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8
Convexity @ Super. elev. Trans.	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8
Defl., Convexity & Super. elev. Trans.	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8

Note: ⊕ Sign denotes "Sag Vertical Curve Effect".
No Camber required for beams in span 1.

FRAMING PLAN



*Note: See Pier 1 details for variation in location of anchor rods.

Note: Trim corner of masonry plate 4 1/2 x 4 1/2 at the following locations:
N.E. Corner Girder A
S.E. Corner Girder K.

GIRDER LENGTHS (c to c Brg.)

Girder	A	B	C	D	E	F	G	H	J	K
Span 1	34'-8 3/8"	34'-10 7/16"	35'-0 11/16"	35'-2 2/8"	35'-5 5/8"	35'-5 13/16"	35'-8 1/8"	35'-10 1/4"	36'-0 3/8"	36'-2 3/4"
Span 3	92'-2 7/16"	91'-0 1/16"	89'-11 1/16"	88'-9 7/16"	87'-8 1/8"	87'-4 1/2"	86'-2 3/8"	85'-1 1/8"	83'-11 5/8"	82'-10 3/8"

DEFLECTION AND CAMBER TABLE FOR SPANS 2 & 3

SPAN	2										3									
	A		B		C		D		E		F		G		H		J		K	
LOCATION (See Diagram)	a	b	a	b	a	b	a	b	a	b	a	b	a	b	a	b	a	b	a	b
Deflection due to Wt. of Steel	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Deflection due to Remaining D.L.	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8
Convexity @ Super. elev. Trans.	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8
Defl., Convexity & Super. elev. Trans.	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8	-1/8
Required Camber ⊕	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8	1/8

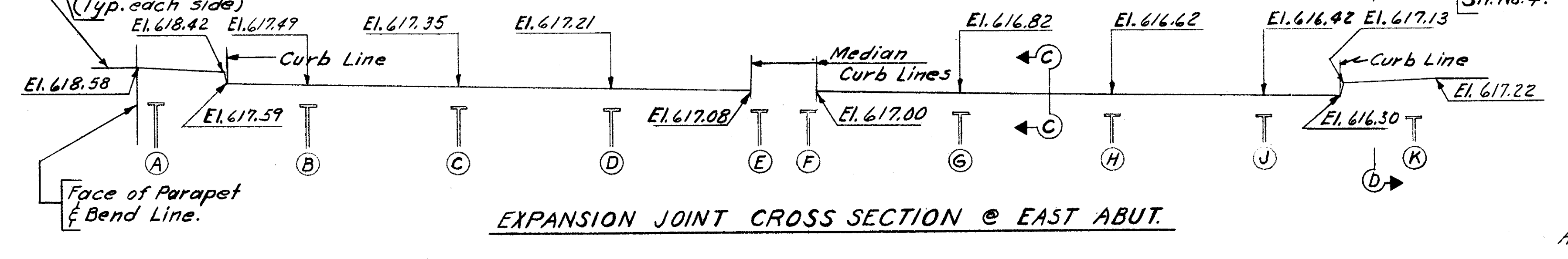
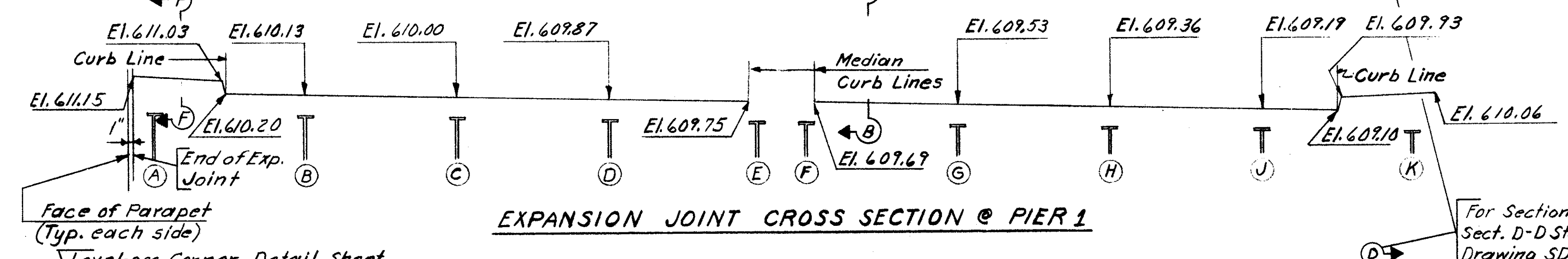
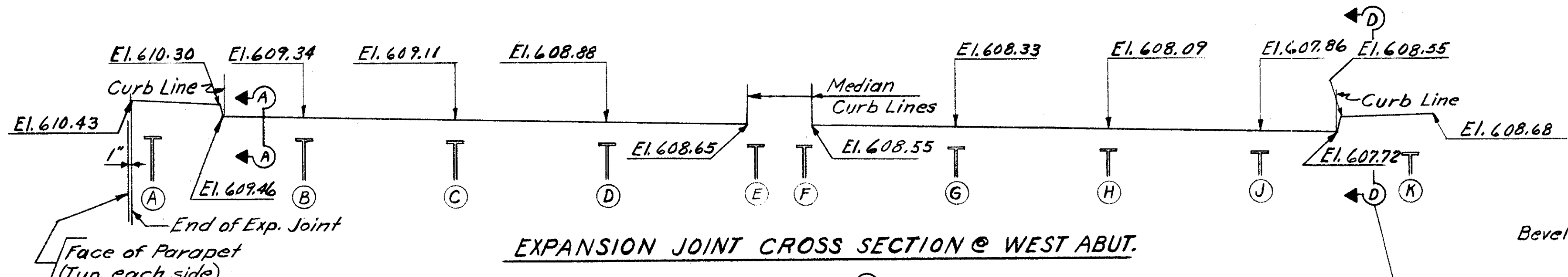
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STRUCTURAL STEEL DETAILS

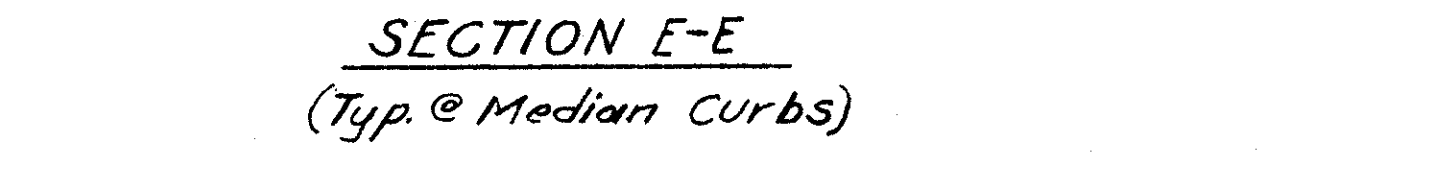
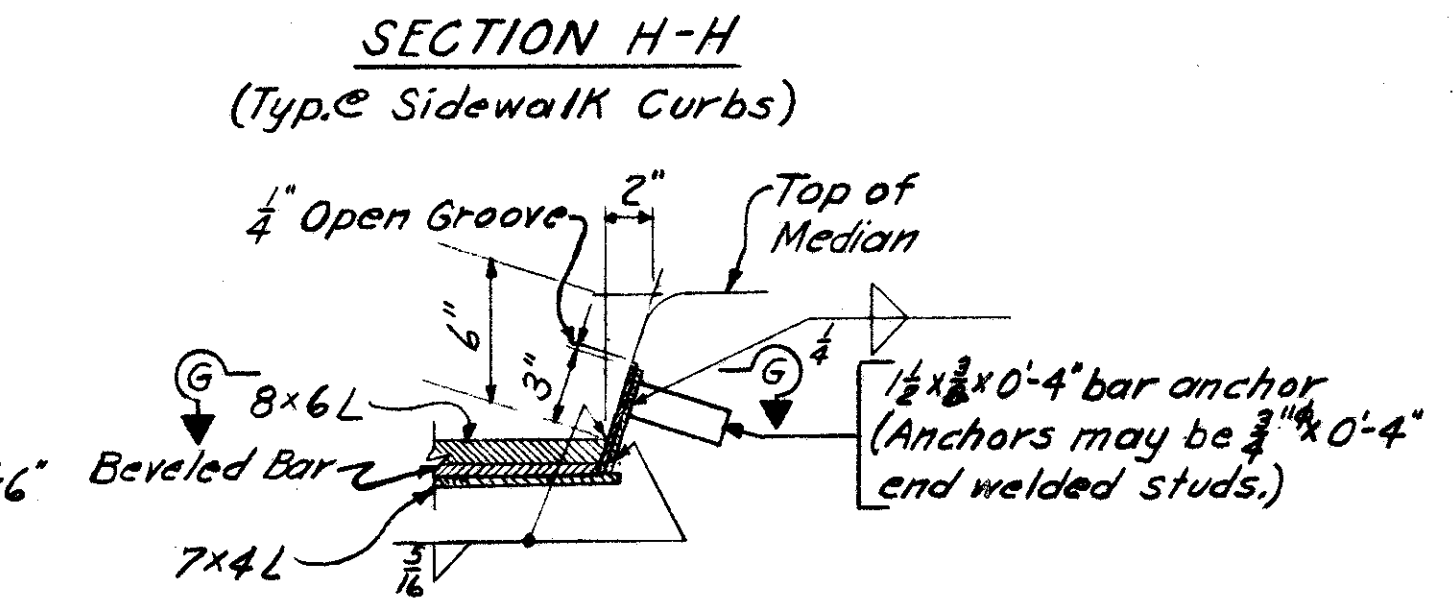
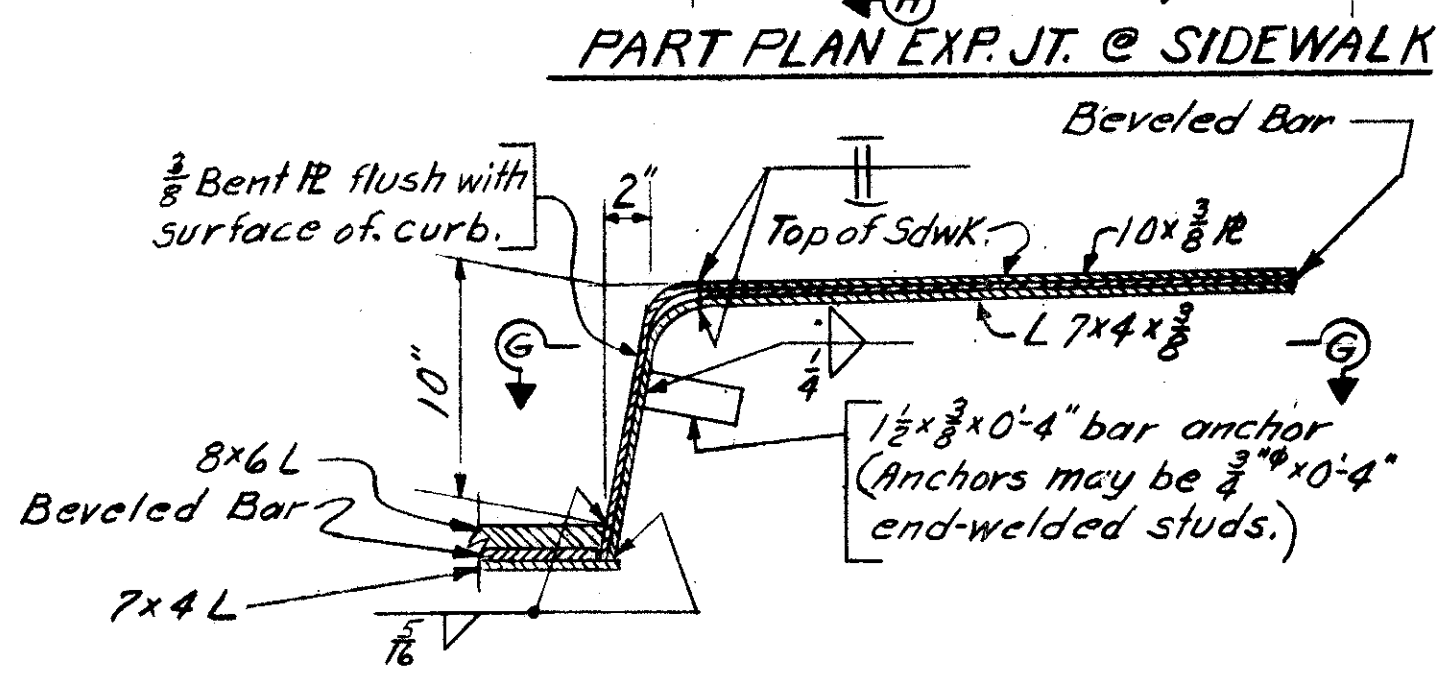
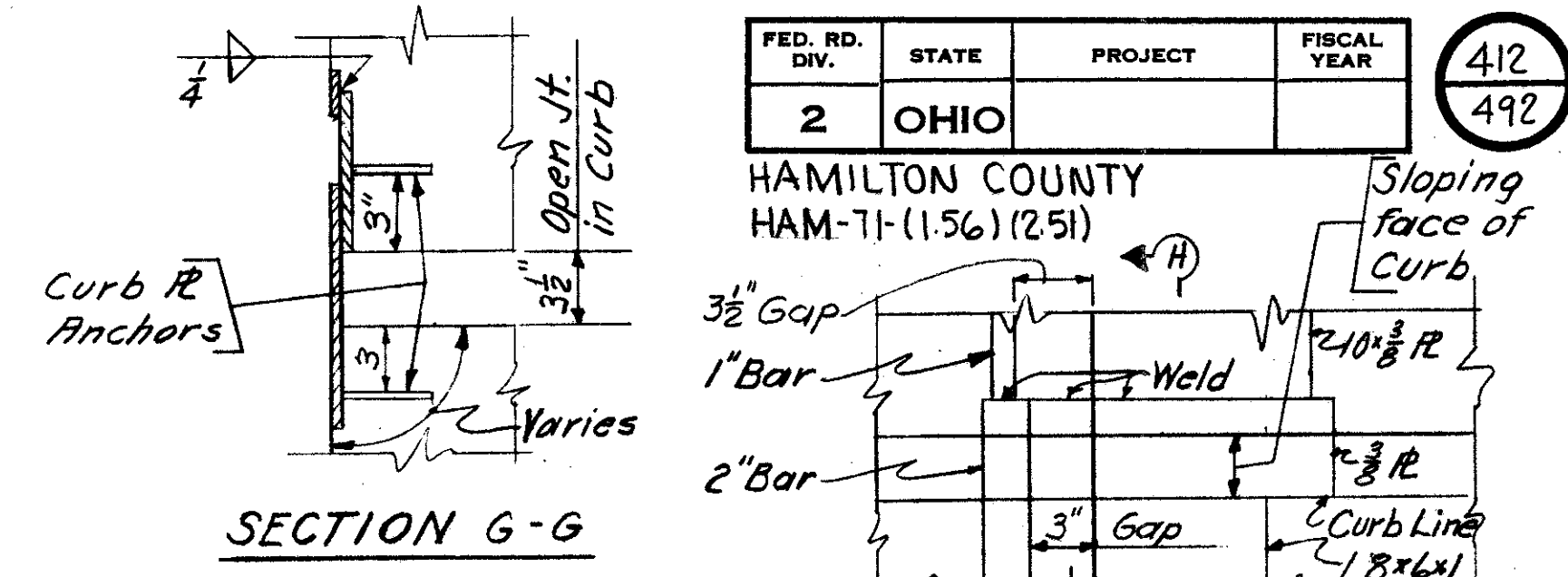
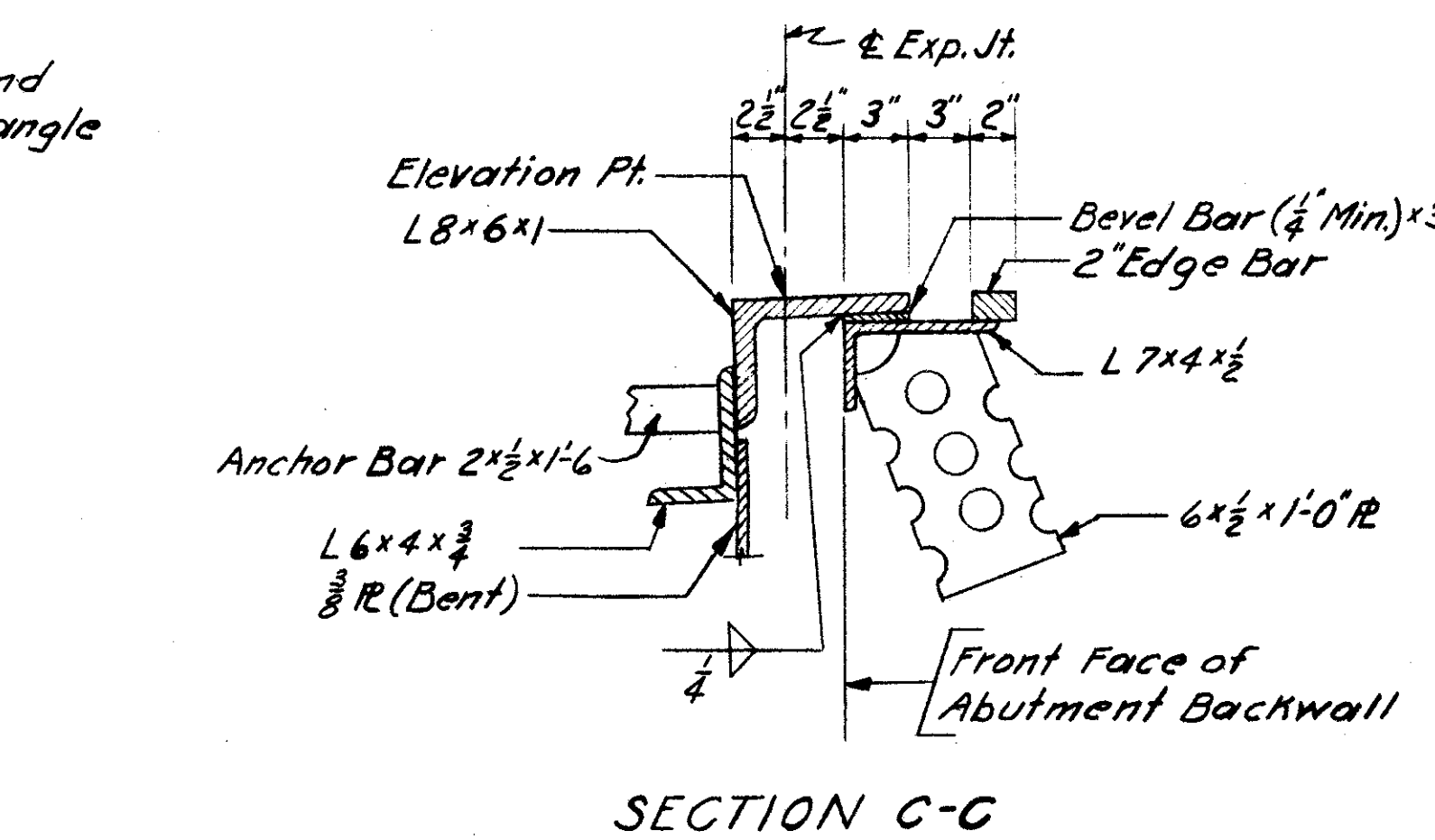
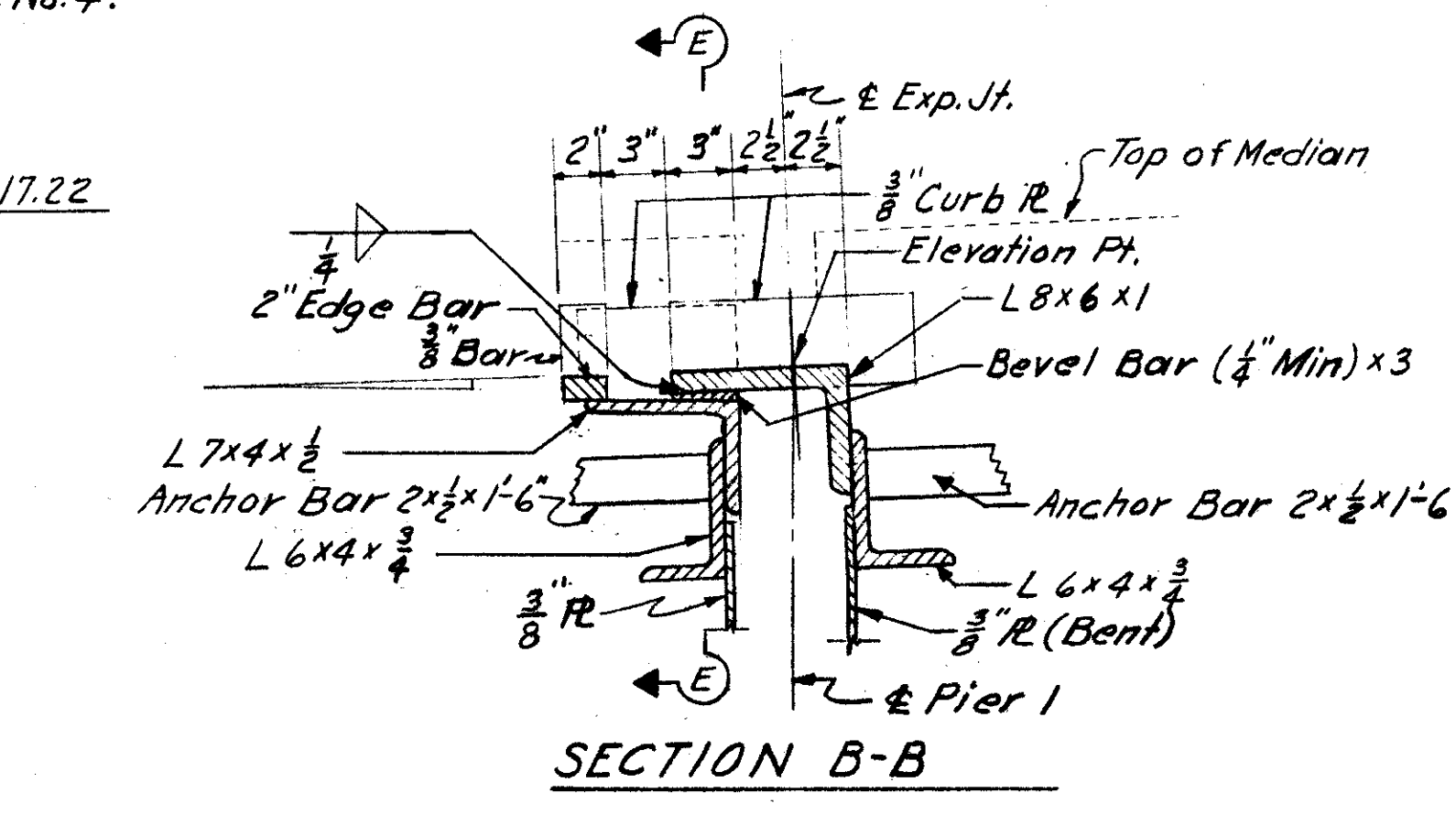
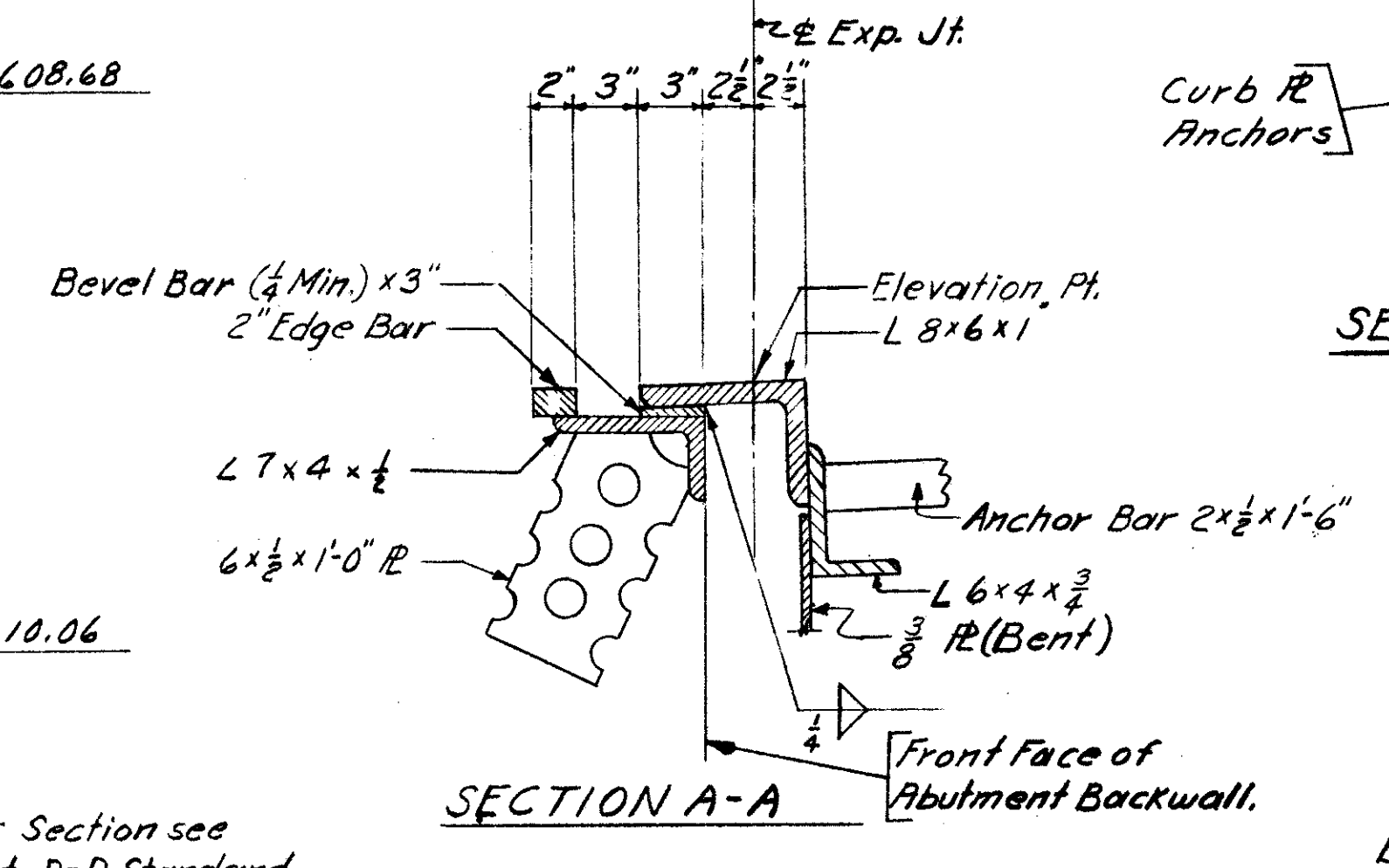
BRIDGE NO. HAM - 71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	H.W.T.		M.L.Z.	J.H.O. 3-19-65	

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FEB 20 1965

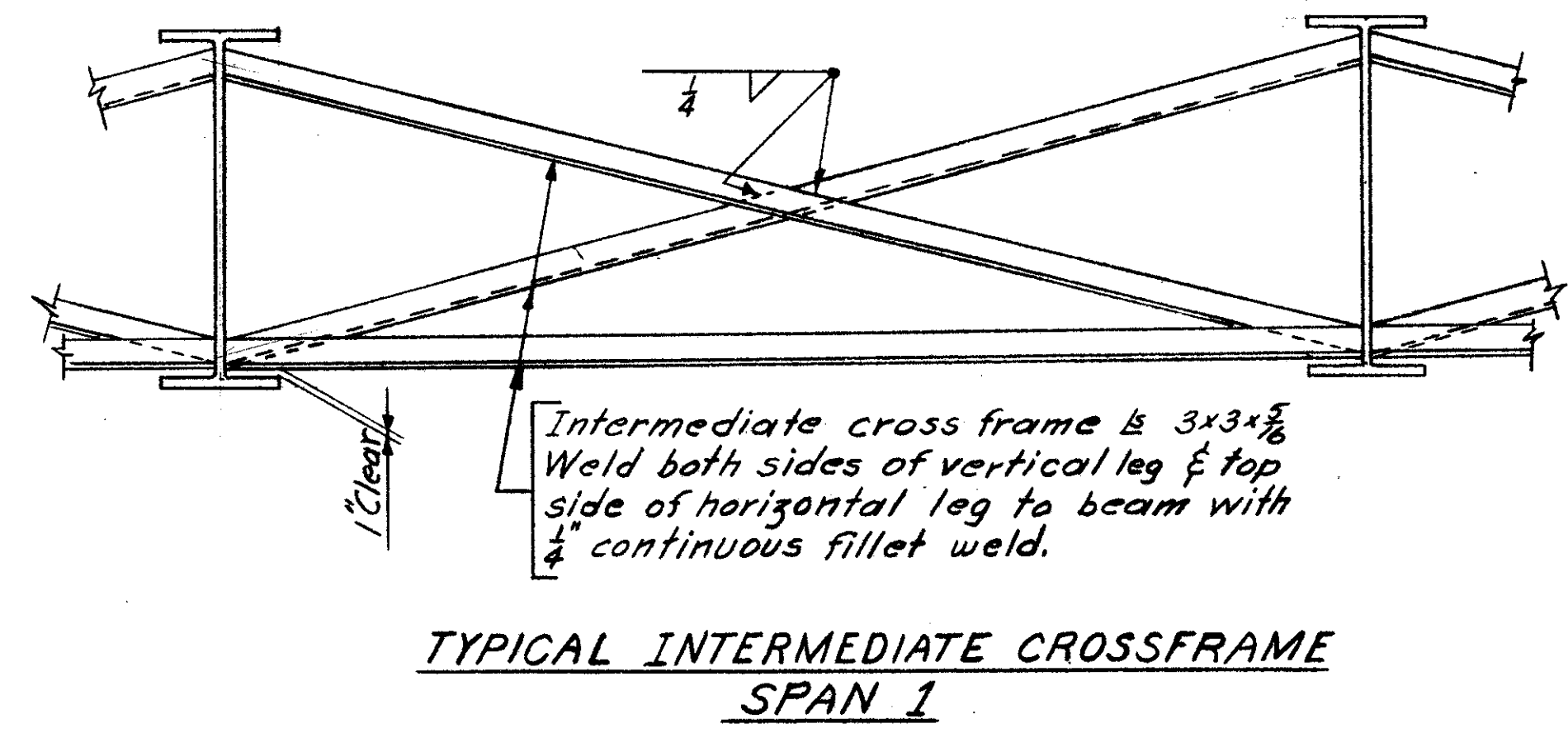
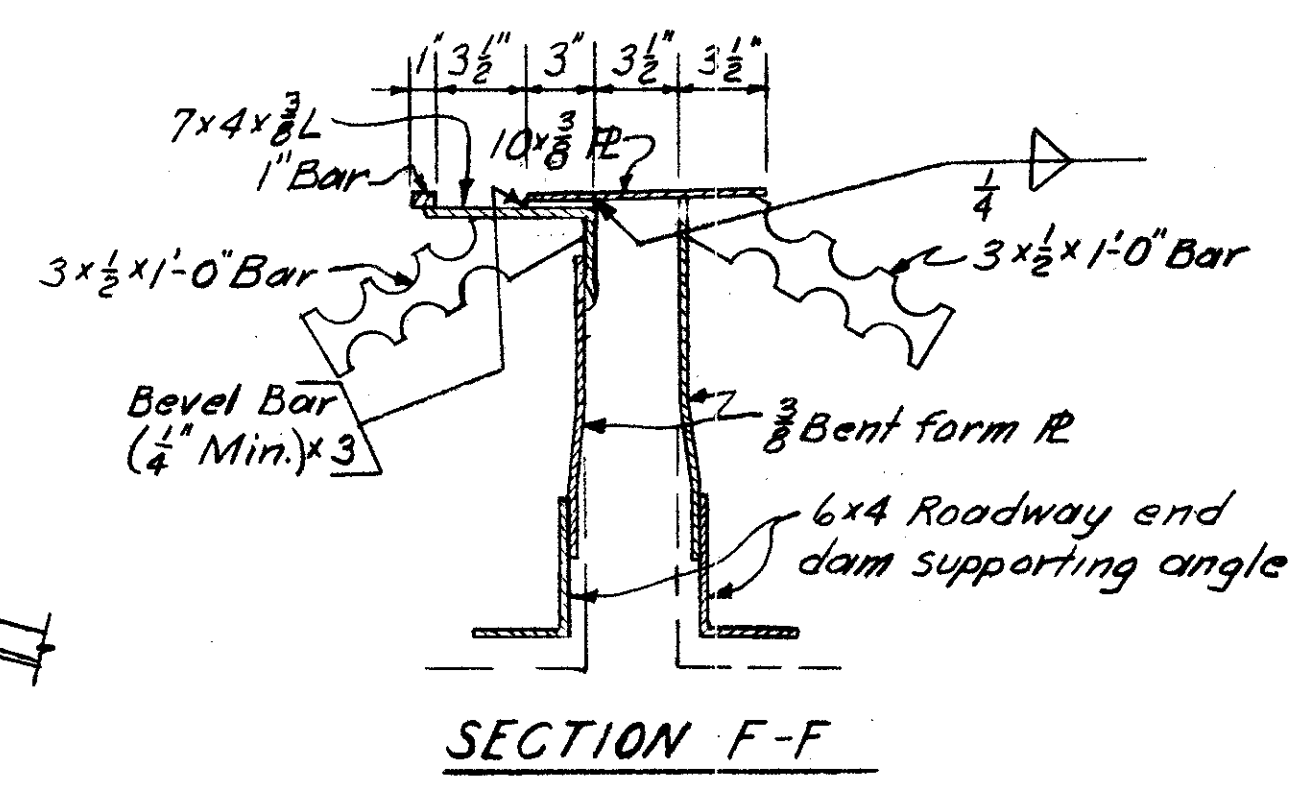


Note: Elevations given are at the intersection of the Girder or Curb Line and the expansion joint (Shown on Sections). Roadway expansion joints are to be bent in shop to conform to contour of top of slab.



Notes: For details not shown see Std. Drwg. SD-1-63, Sheets 2 & 4 and Typical Structural Steel Details, Sheet No. 444. For details of bearings see Std. Drawings. FSB-1-62.

For Inlet Framing details see drainage details. Expansion joint openings shown on details are for a normal temperature of 70°F.

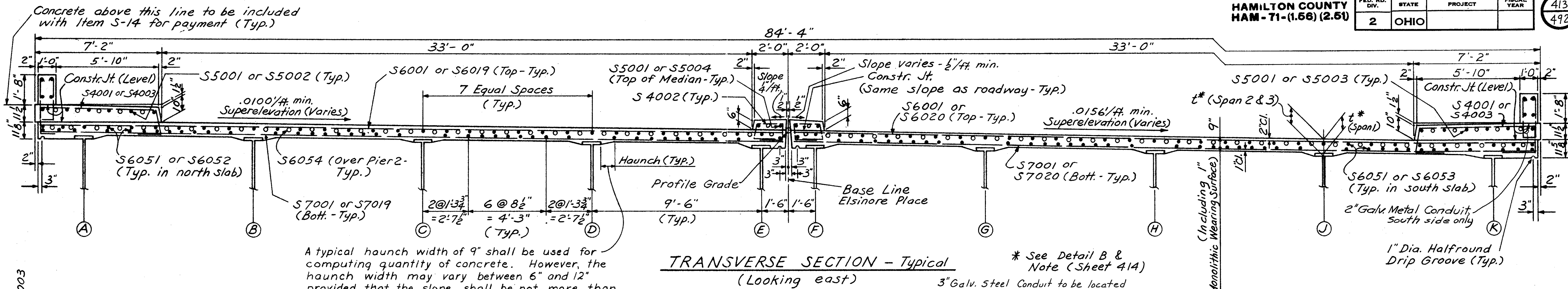


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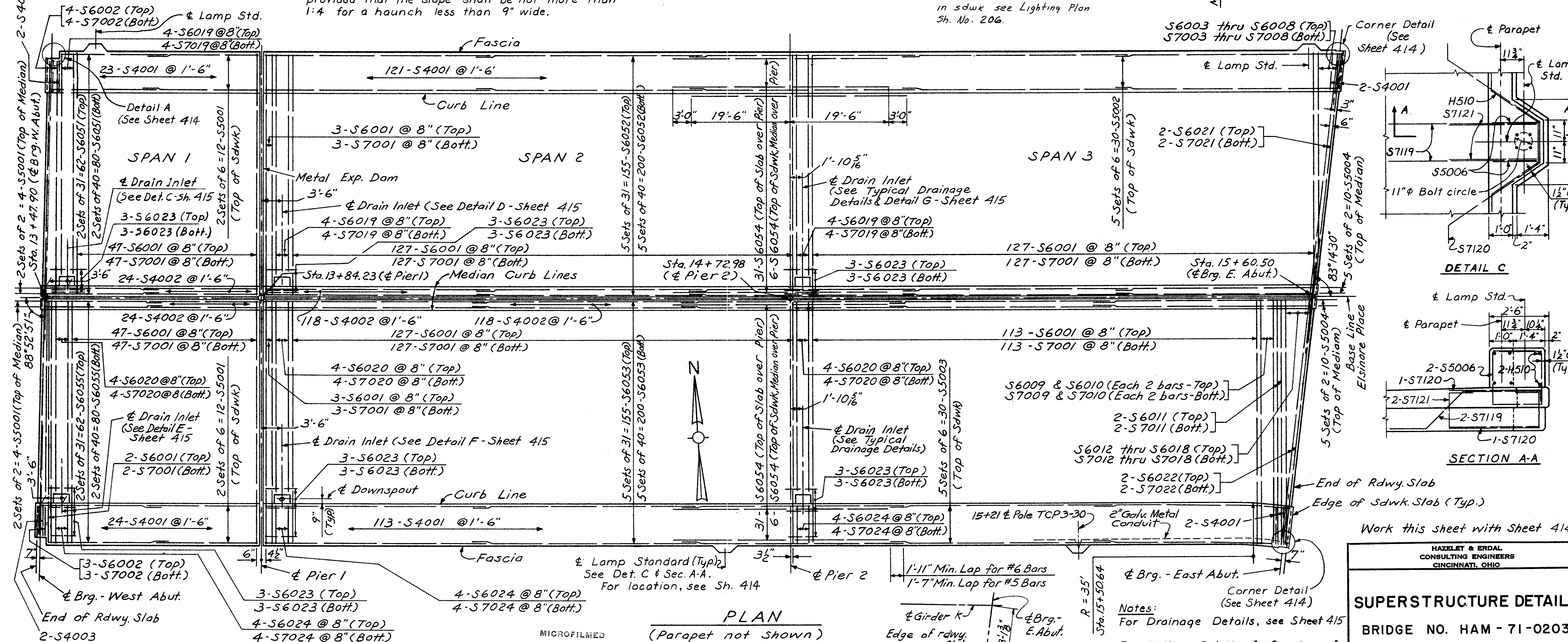
STRUCTURAL STEEL DETAILS
BRIDGE NO. HAM - 71-0203

MICROFILMED
FEB 20 1985

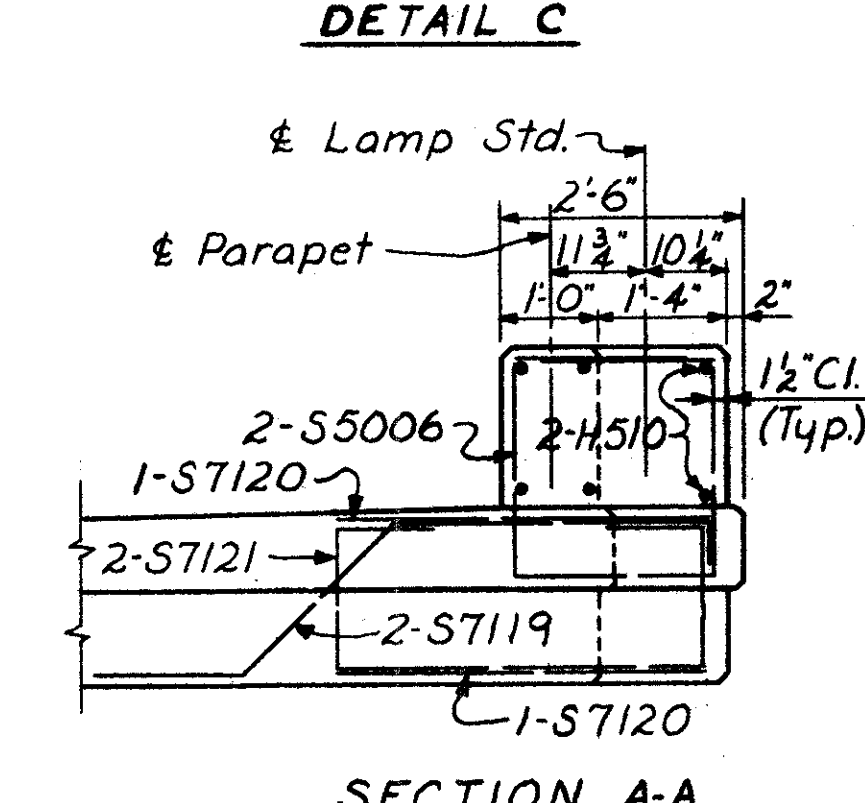
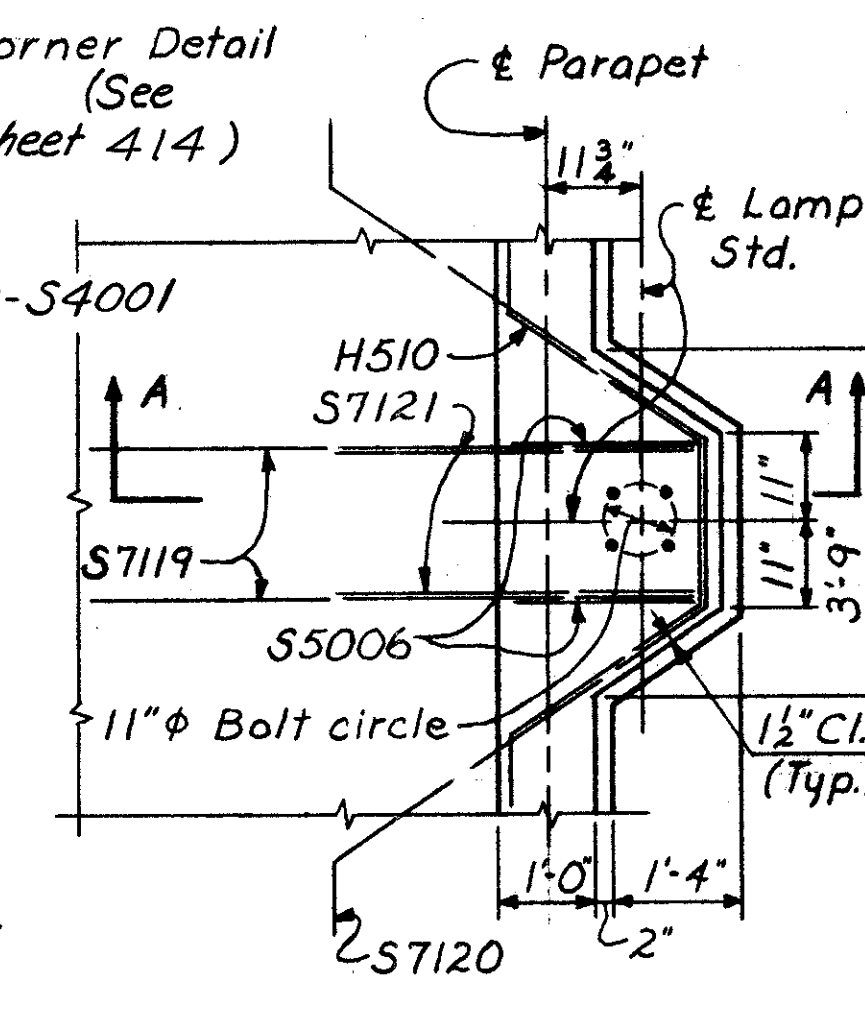
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	H.W.T.		W.J.Z.	J40 5/20/65	



TRANSVERSE SECTION - Typical
(Looking east)



PLAN
(Parapet not Shown)



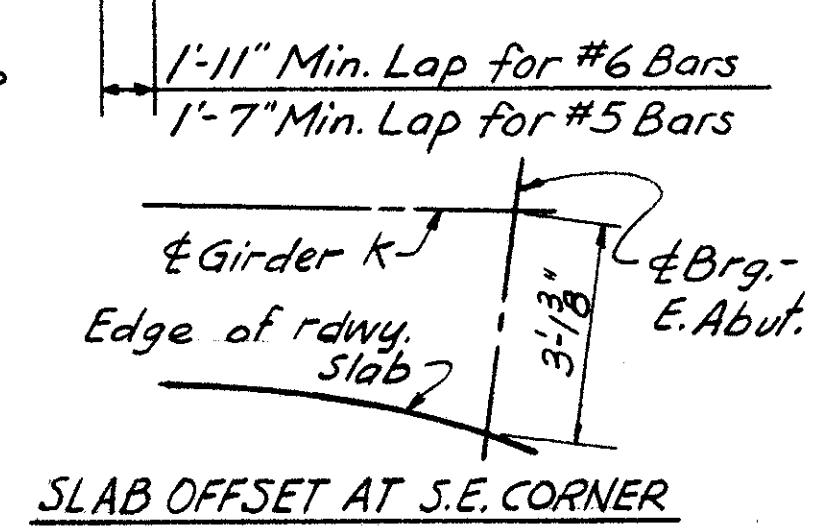
SECTION A-A

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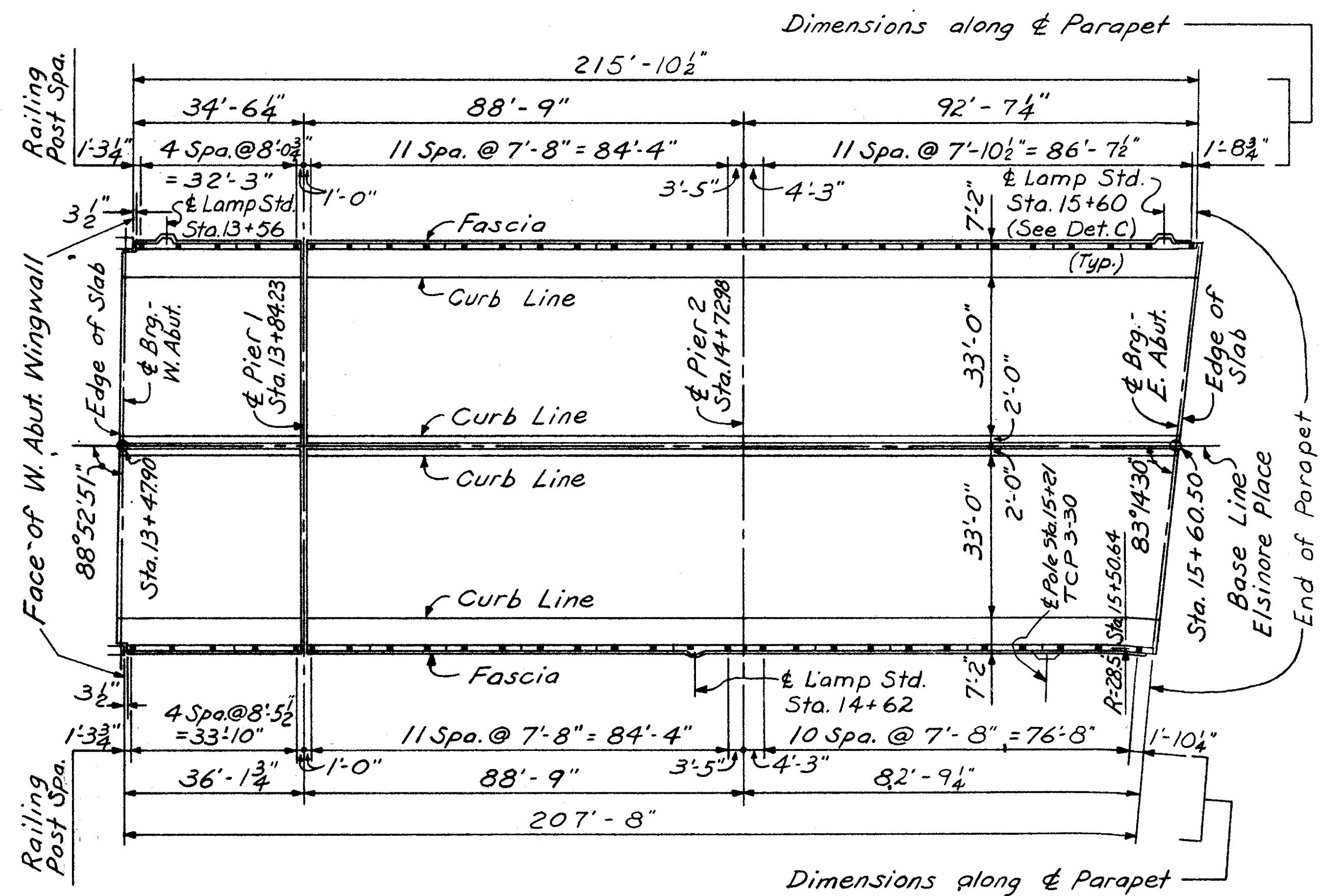
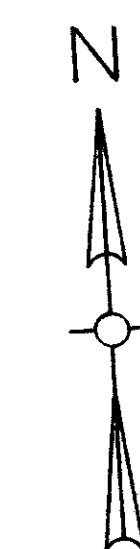
SUPERSTRUCTURE DETAILS
BRIDGE NO. HAM-71-0203

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
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			3-19-65	5/20/65	

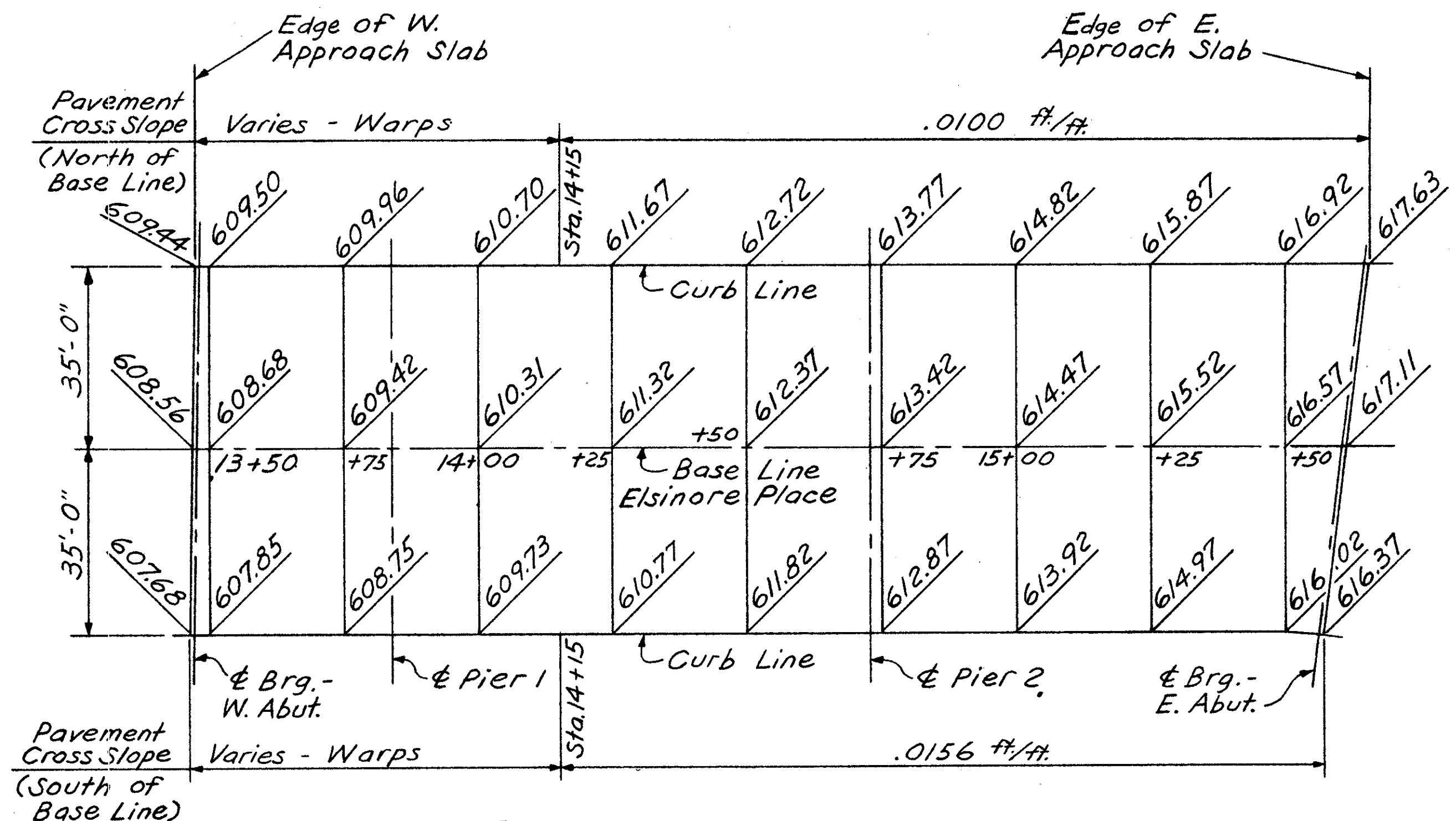
MICROFILMED
FEB 20 1965



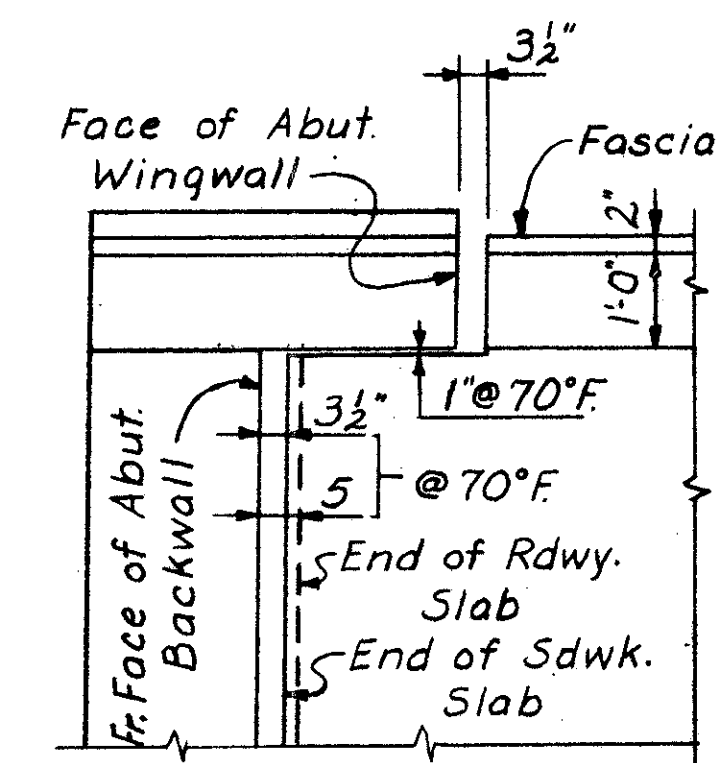
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



PLAN - RAILING

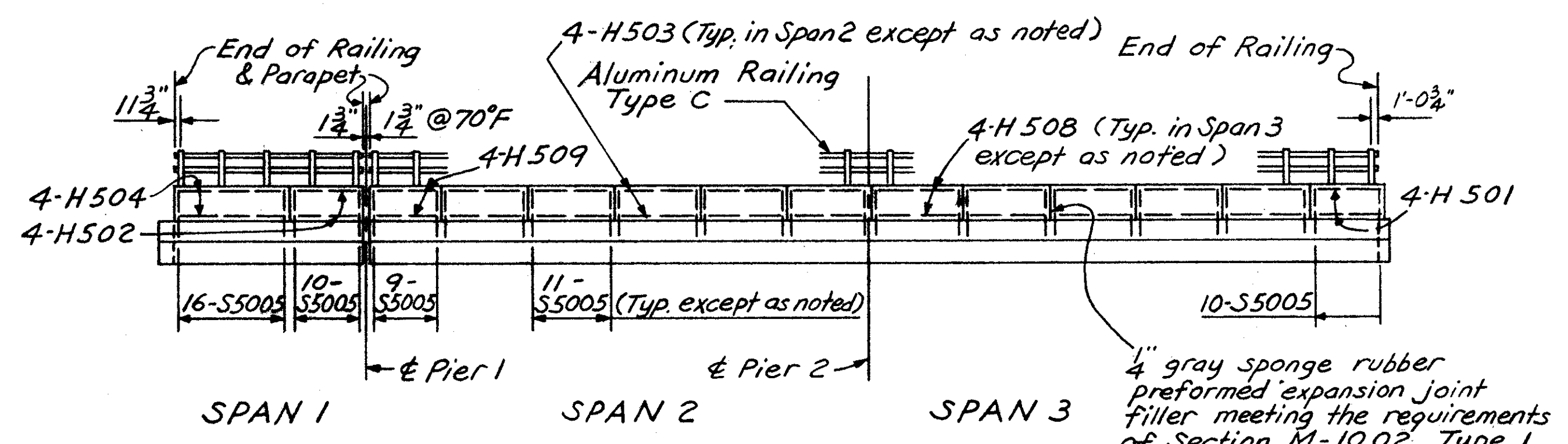


ROADWAY ELEVATIONS

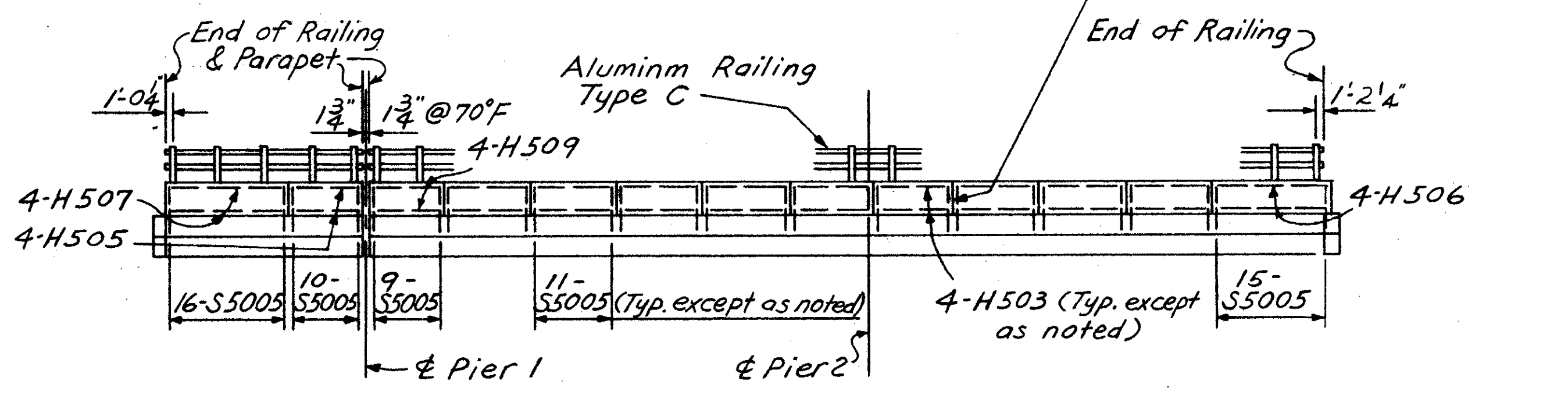


DETAIL A

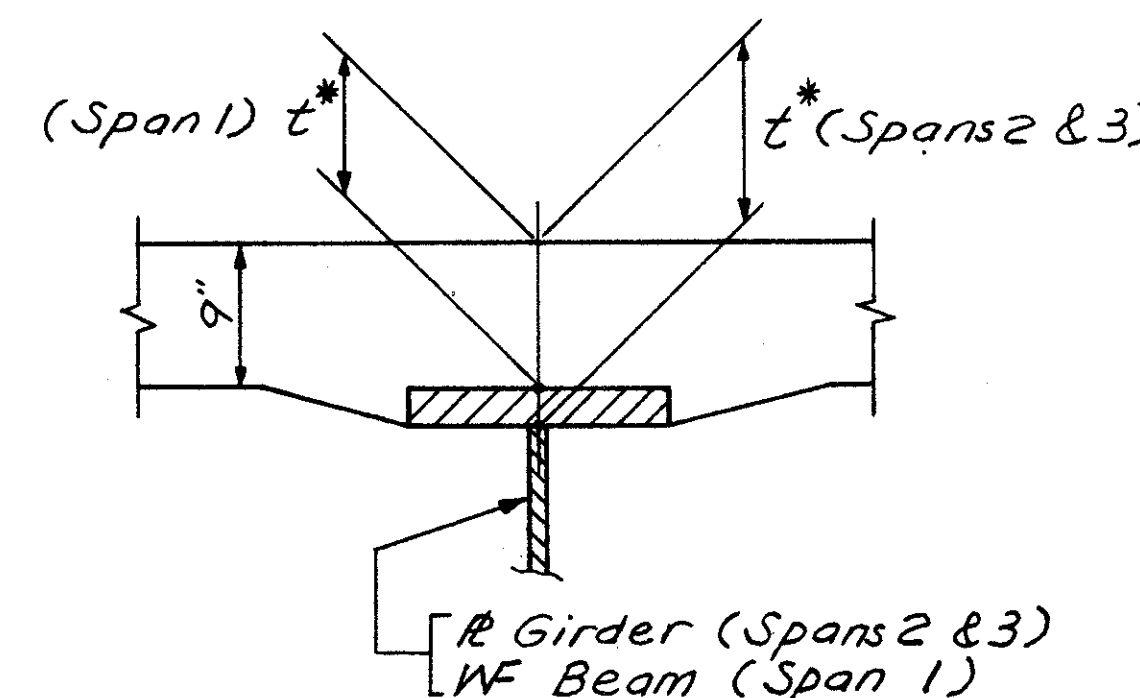
N.W. Quadrant
(S.W. Quad. similar, opp. hand)



ELEVATION - NORTH RAILING



ELEVATION - SOUTH RAILING

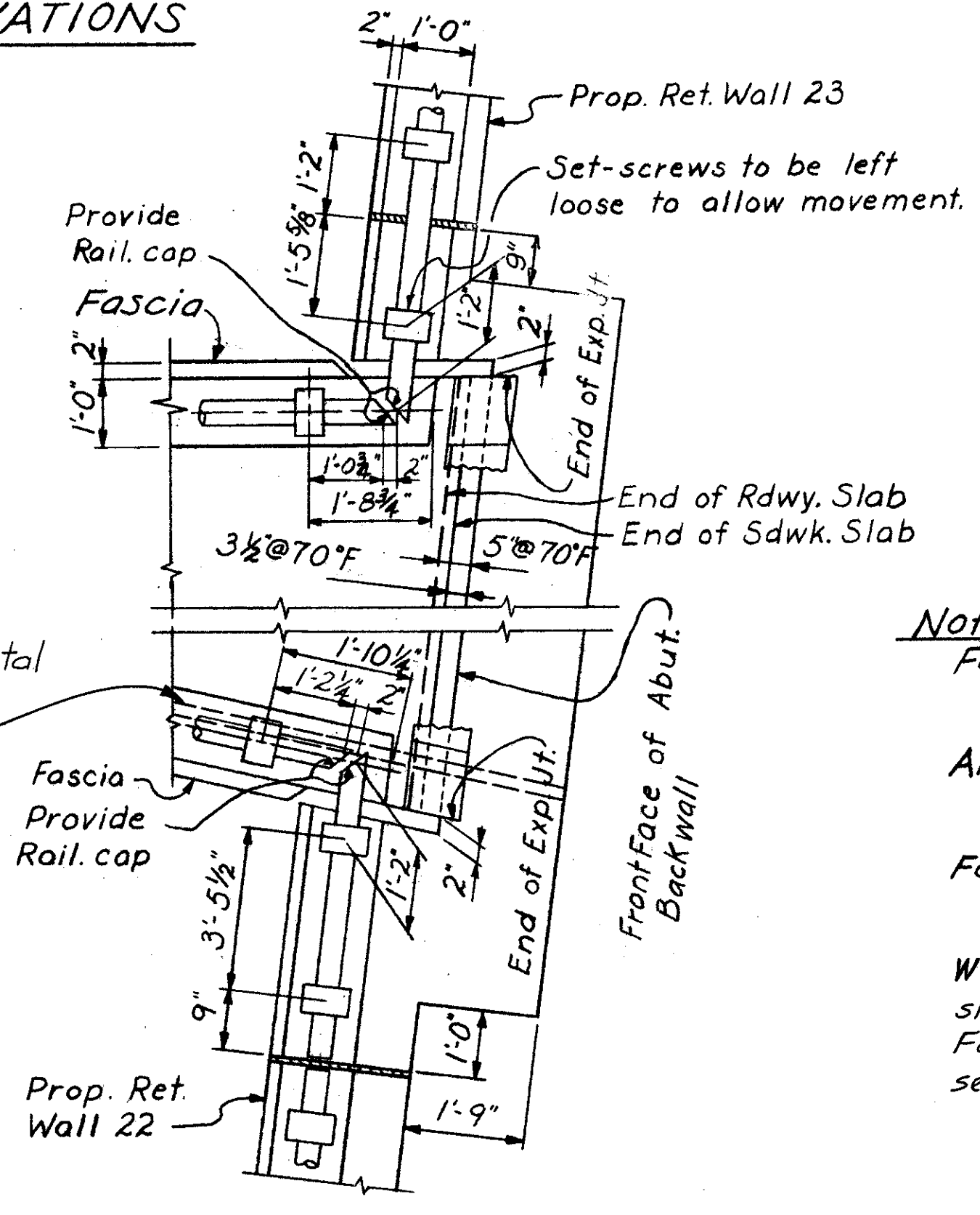


DETAIL B

Girder	Span 1		Spans 2 & 3
	Brig.	Span	
A	10 1/2"	9"	10 3/4"
B	10	9	10 3/4"
C & D	10	9 1/2	10 3/4"
E & F	10	9 1/2	10 3/4"
G	10	9 1/2	10 3/4"
H	10	9 1/2	10 3/4"
J	10	9 1/2	10 3/4"
K	10 1/2	10 1/4	11 1/2

Note
Slab thickness includes 1" monolithic wearing surface.

* This is the nominal dimension.
The quantity of deck concrete to be paid for shall be based on this dimension, even though deviation from it may be necessary because the top flange of the beam may not have the exact camber or conformation required to place it parallel to the finished grade. Deduction shall be made for volume of encased steel plates as per Sec. S-1.25 of the Construction and Material Specification.



CORNER DETAIL
(N.E. & S.E. Quadrants)

Notes
For railing details not shown, see Standard Drawing No. AR-1-57.
Aluminum tube railing should be bent to the curvature of the E of Parapet.
For Bridge Lighting Details, see Sheet 216.
Work this drawing with Lighting Plan sh no. 206
For conduit expansion detail at abutments see Sh. no. 214.

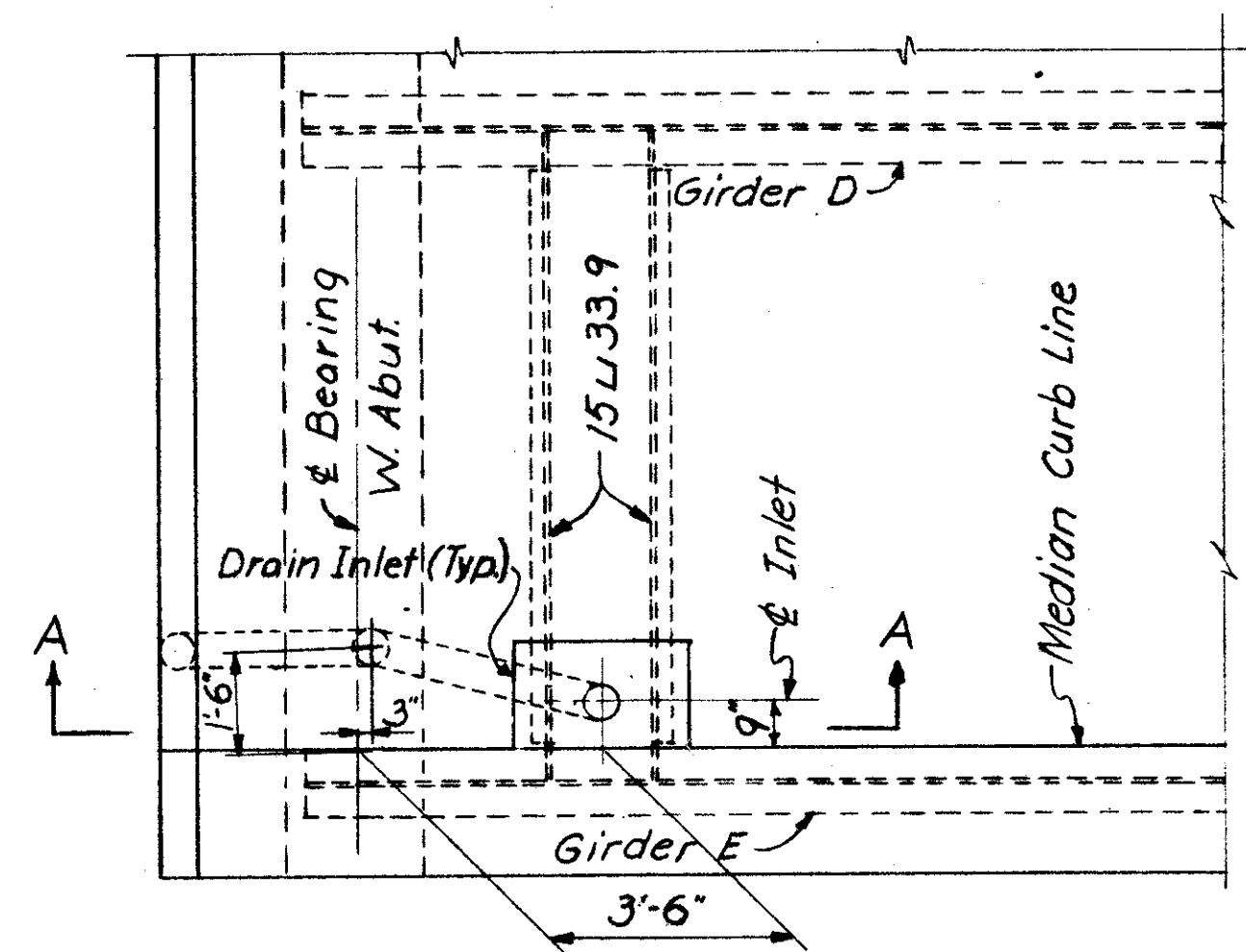
Work this sheet with Sheet 413

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
RAILING DETAILS				
BRIDGE NO. HAM - 71-0203				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	GW		D.W.	JH0
			3-19-65	5/20/65

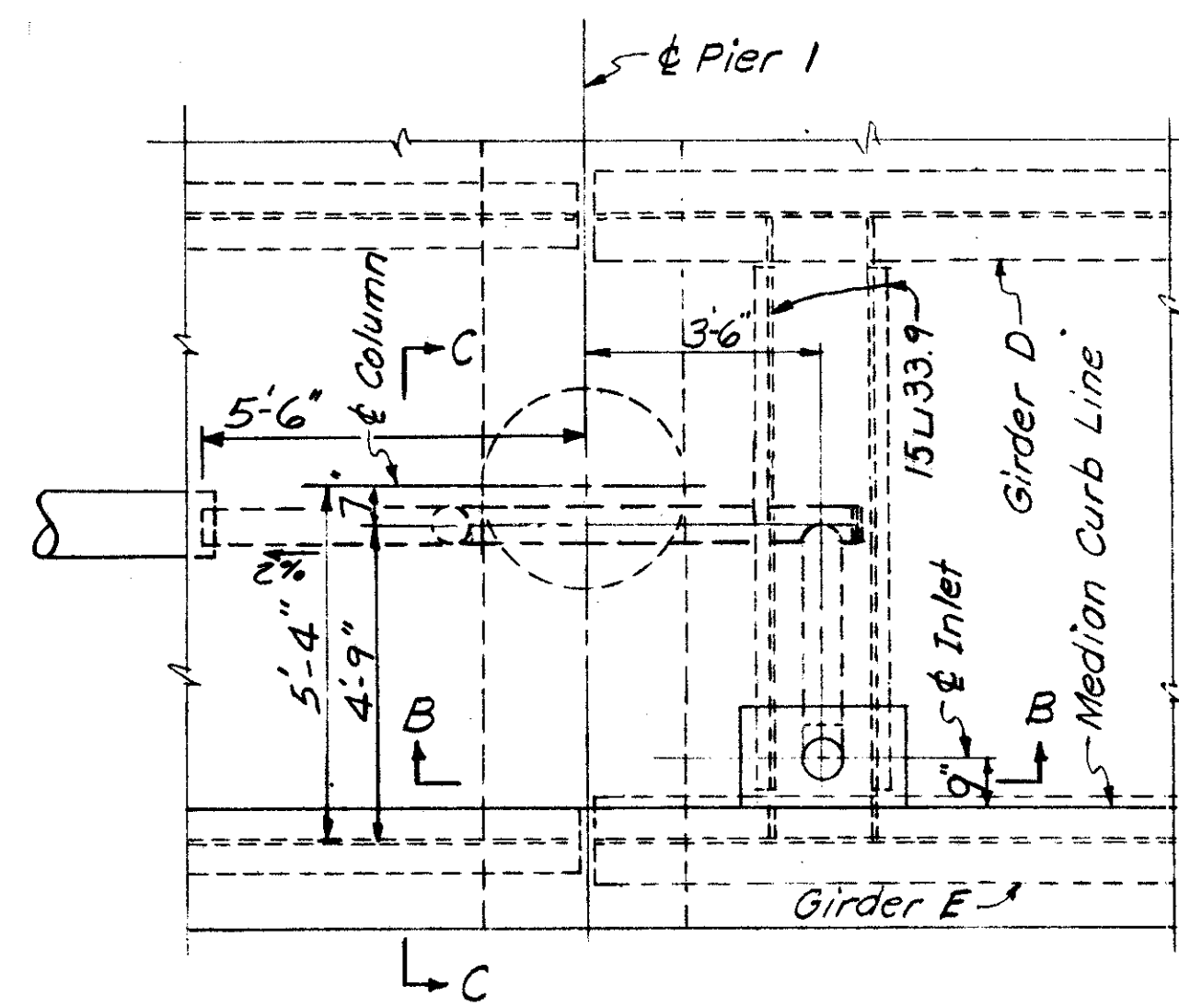
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FEB 20 1965

**HAMILTON COUNTY
HAM-71-(1.56) (2.51)**

Provide Concrete Insert and 1/2" ϕ Rod Hanger with 2" x 1/2" Galv. Pipe Clamp (For detail see Typical Drainage Details, Sh. No. 445)

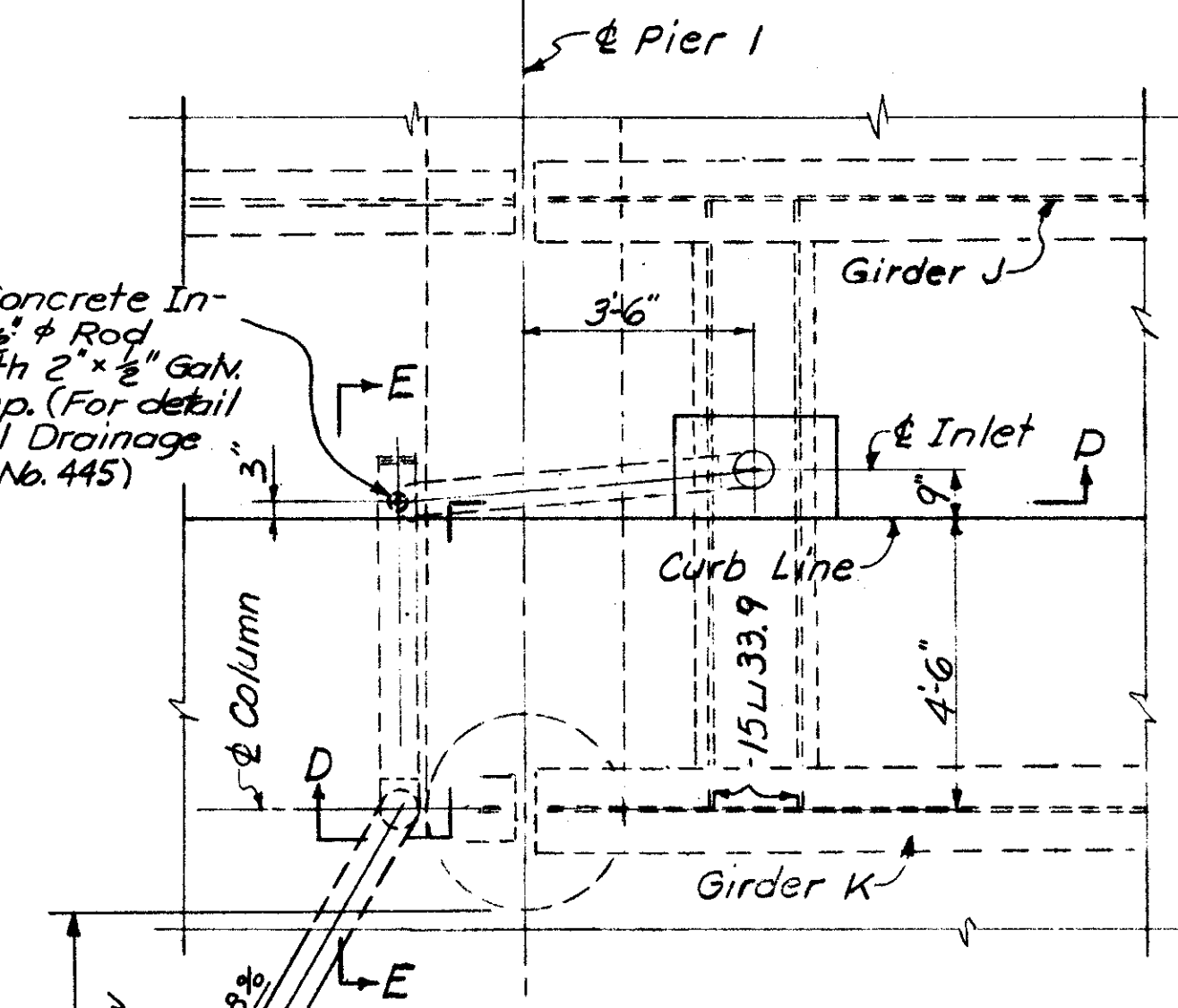


**DETAIL C
DETAIL E SIMILAR**

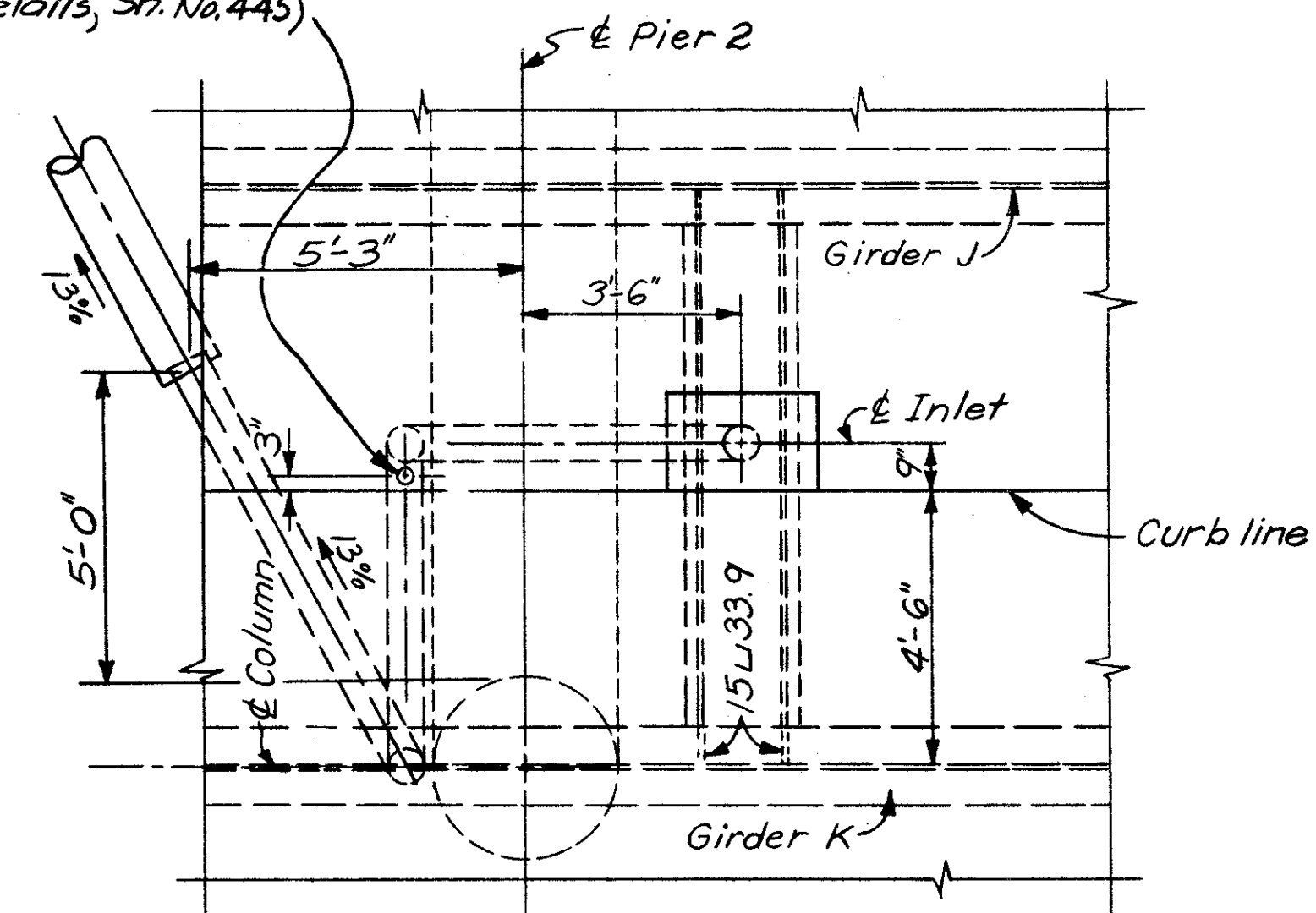


DETAIL D

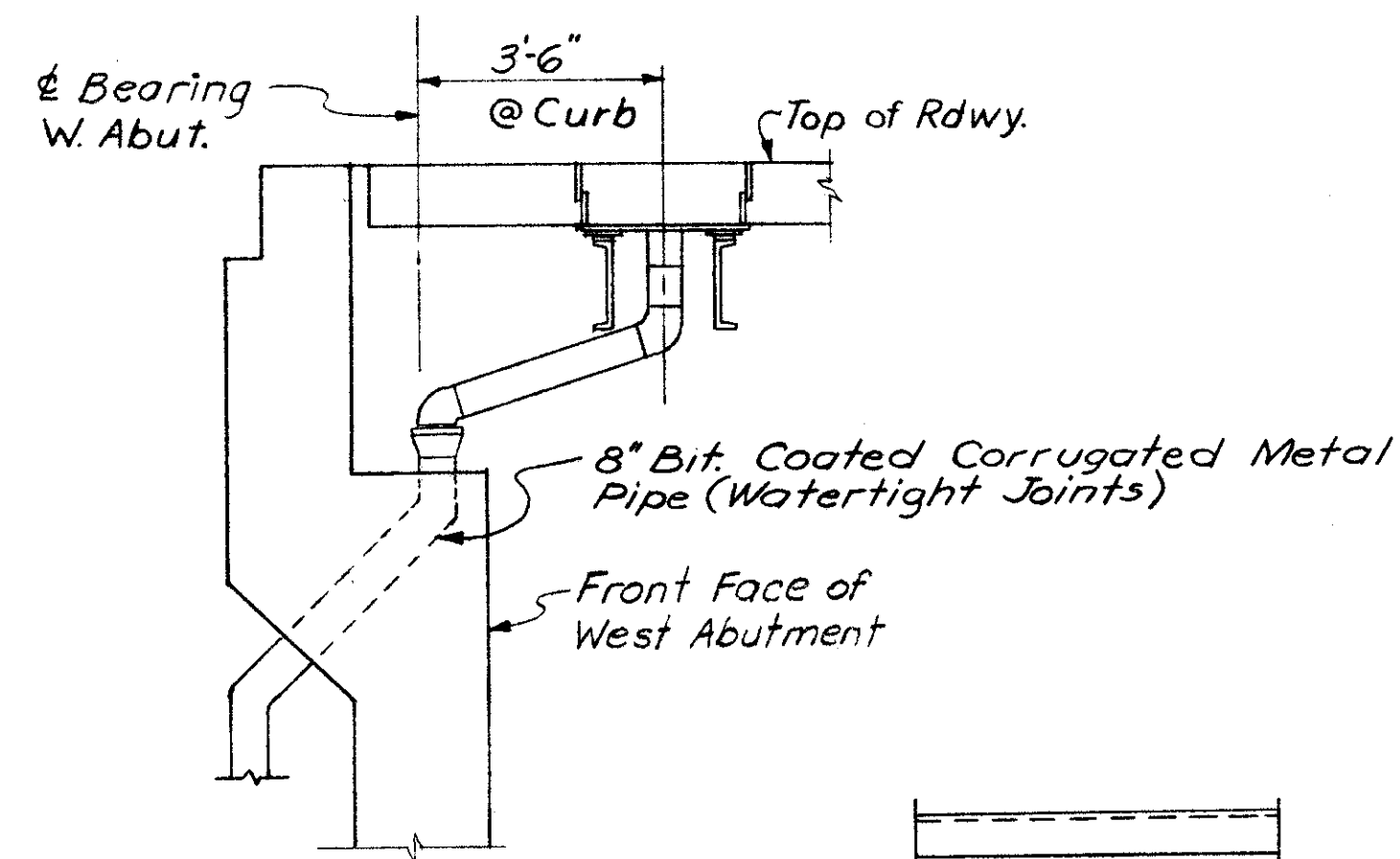
Provide Concrete Insert and 1/2" ϕ Rod Hanger with 2" x 1/2" Galv. Pipe Clamp (For detail see Typical Drainage Details, Sh. No. 445)



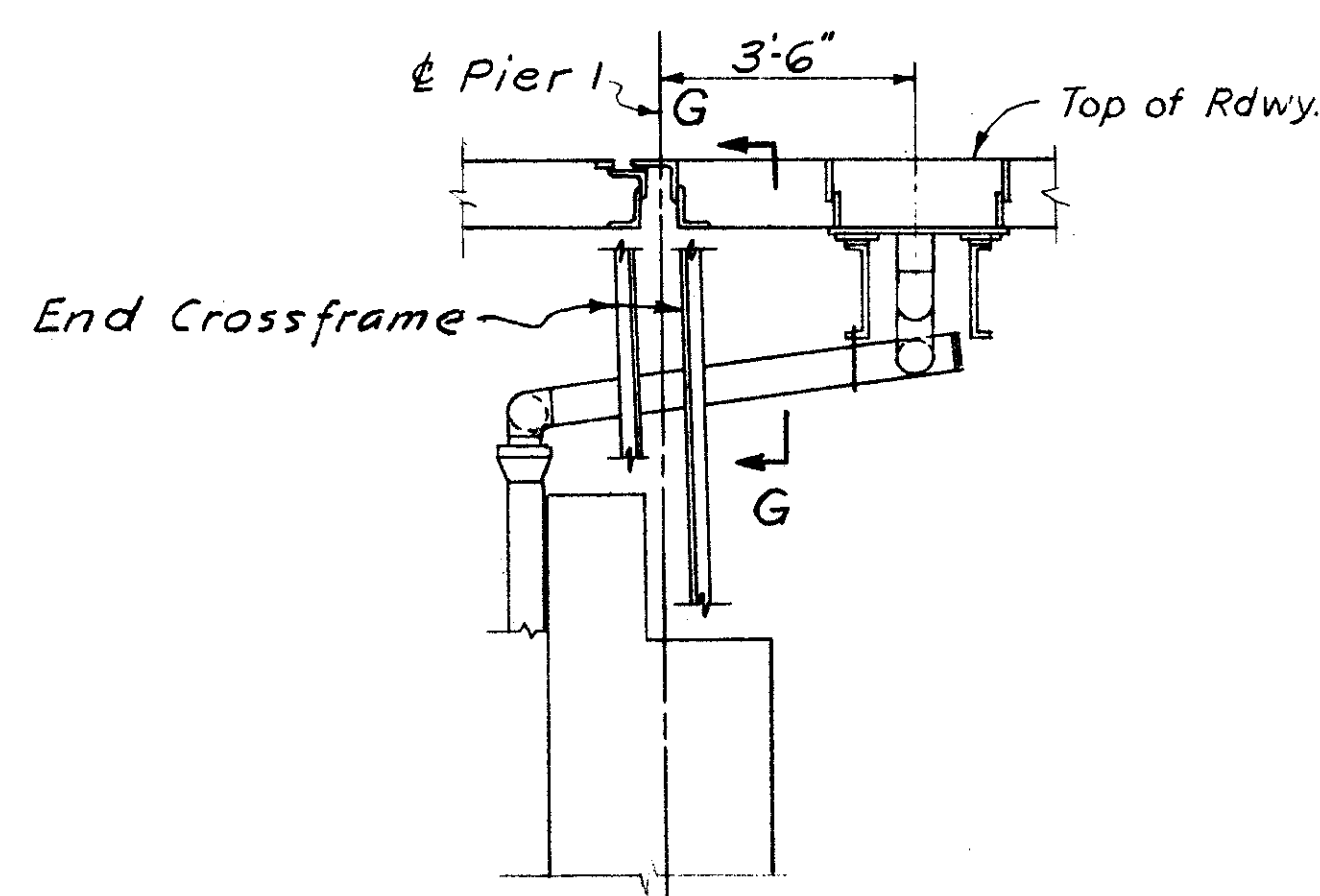
DETAIL F



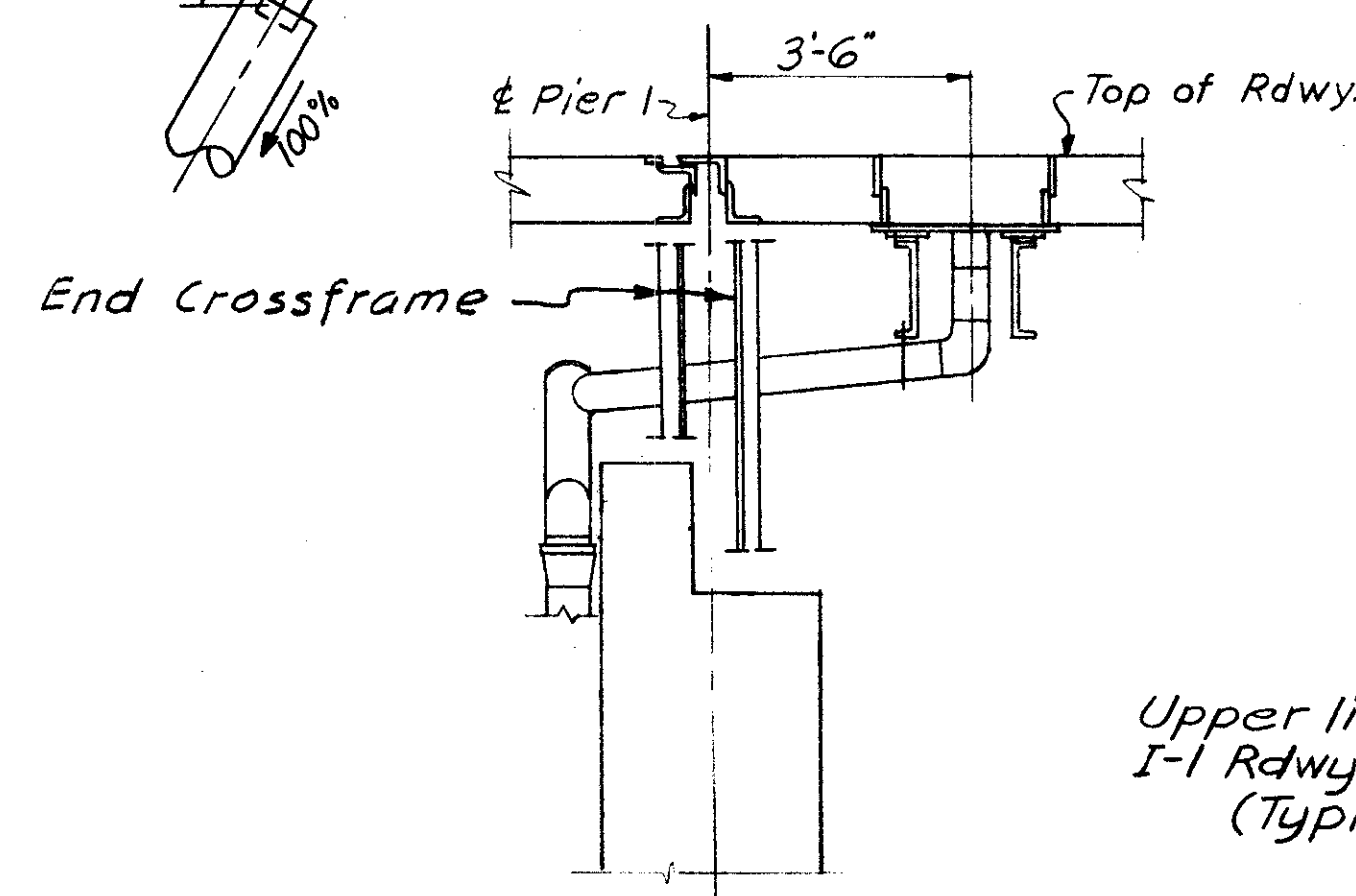
DETAIL H



SECTION A-A



SECTION B-B



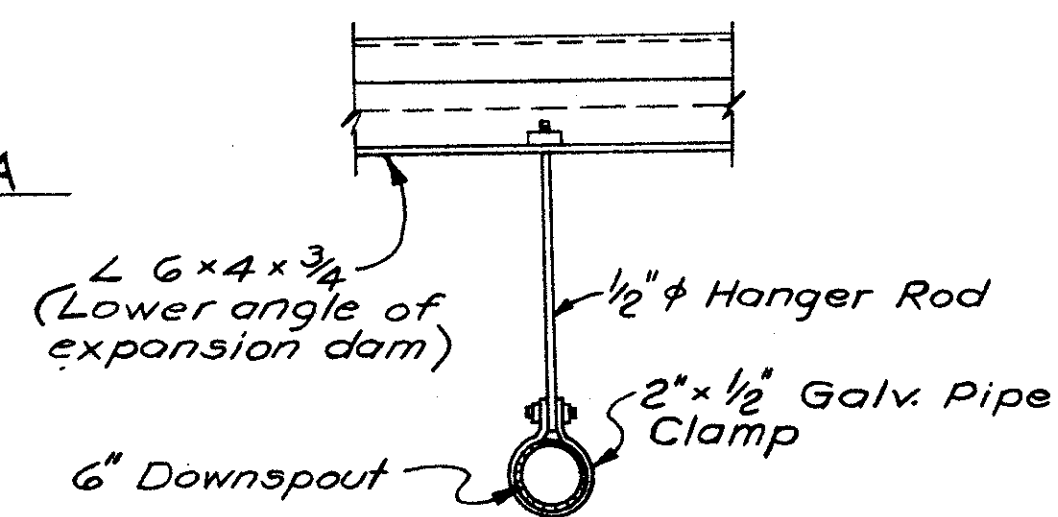
SECTION D-D

Upper limit of Item I-1 Rdwy. Quantities (Typical)

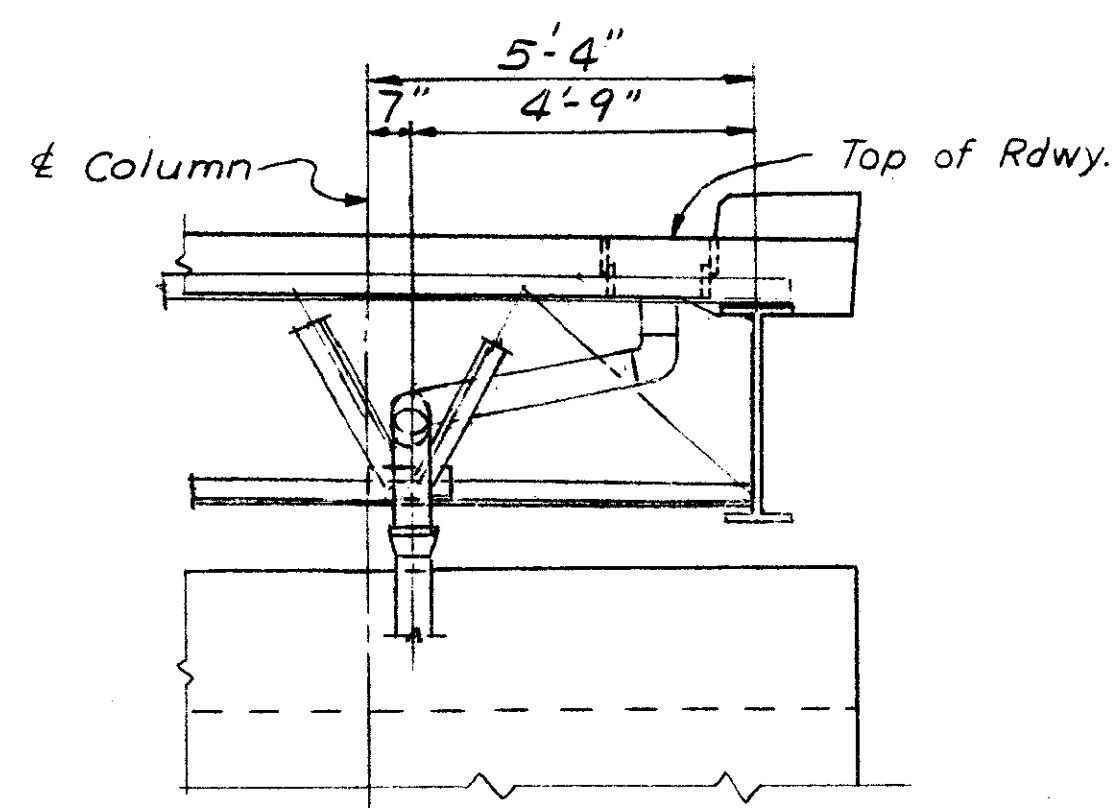
Lower limit of Item 5-29 (Downspout) (Typ.)

For flow line elevation see Roadway Drainage Drawing Sh. No. 95 thru 97 (Typ.)

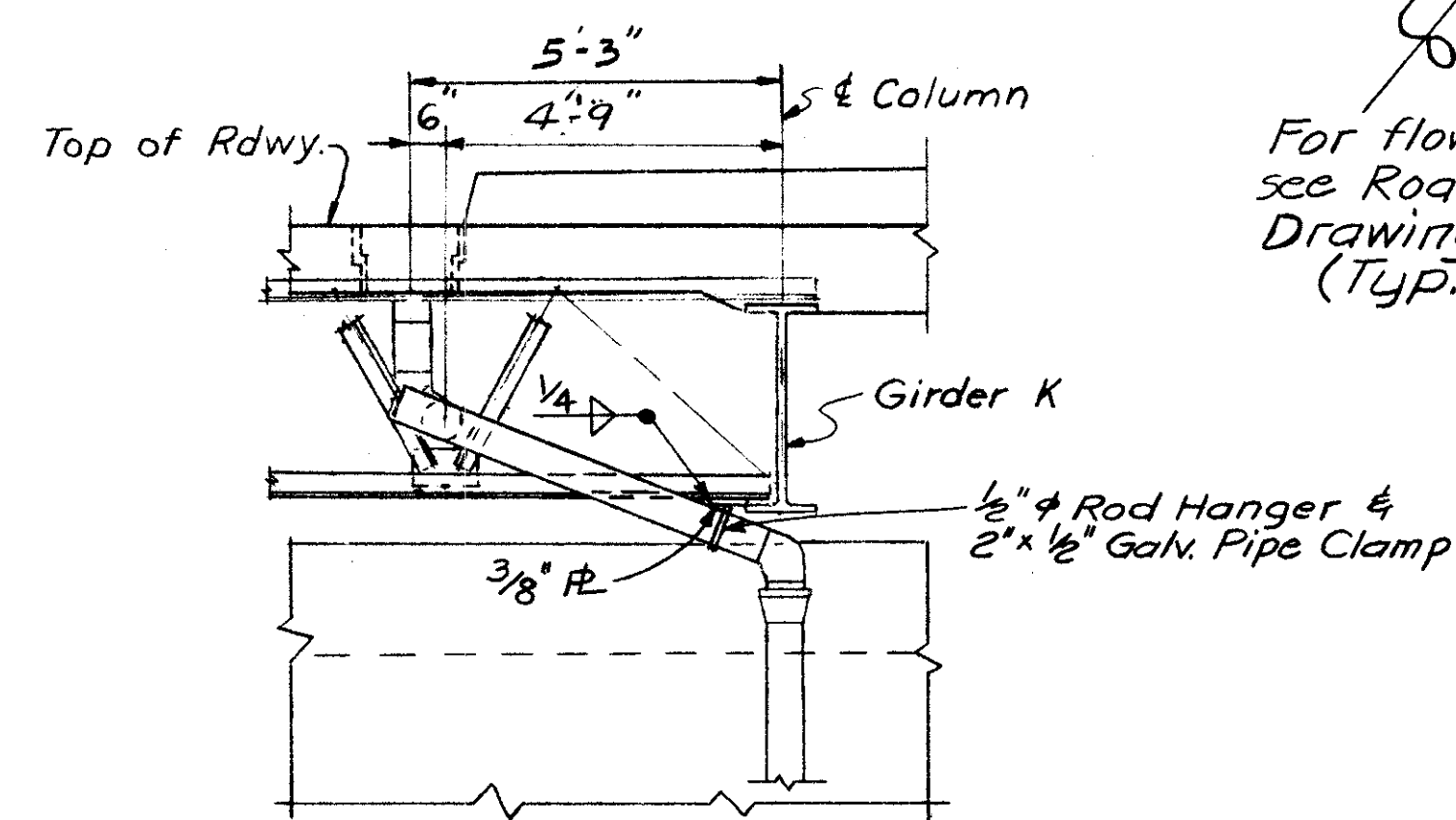
Note: For details not shown, see Typical Drainage Details. Sh. No. 445
6" Pipe shall be used for all downspouts.



SECTION G-G



SECTION C-C



SECTION E-E

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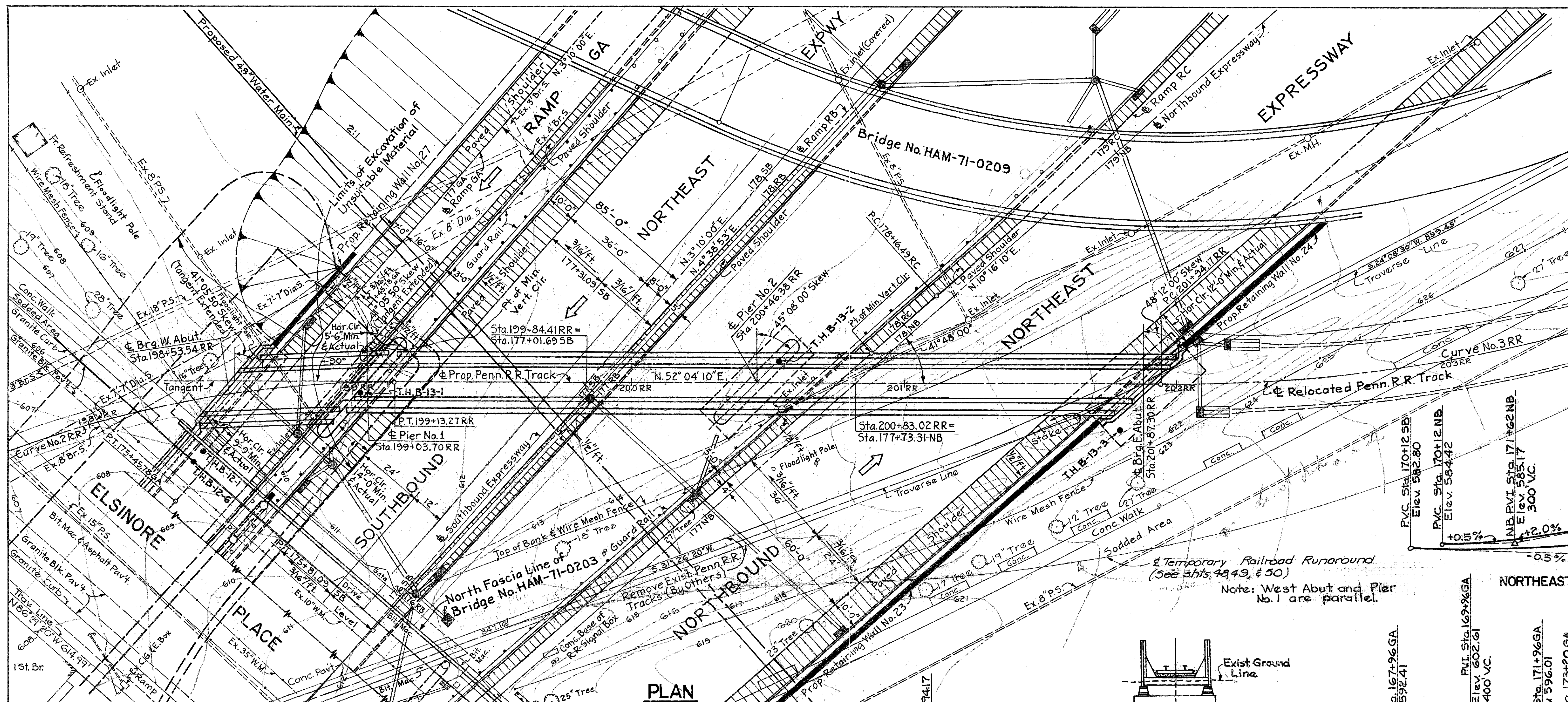
Work this Sheet with Sheet 413

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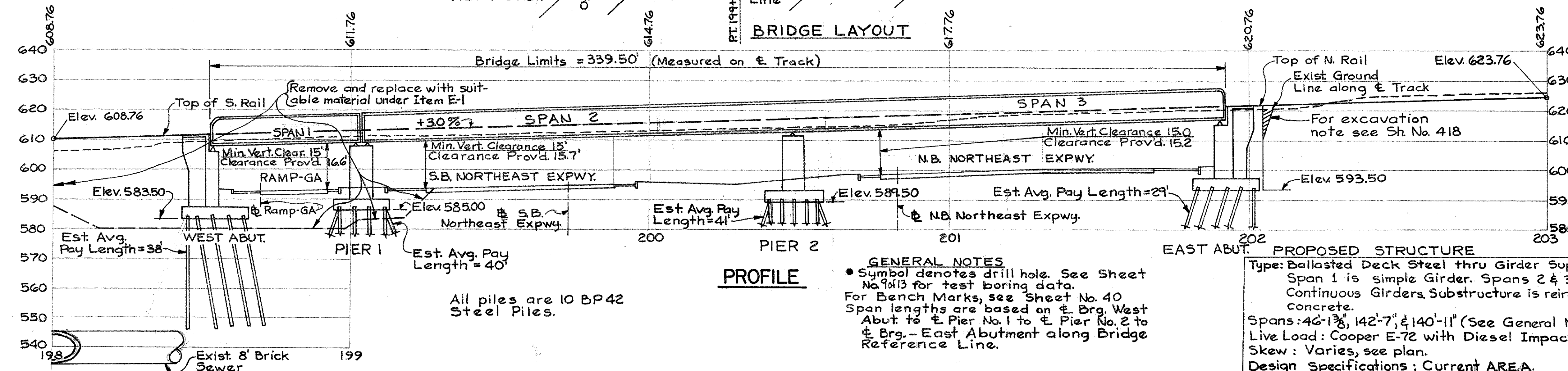
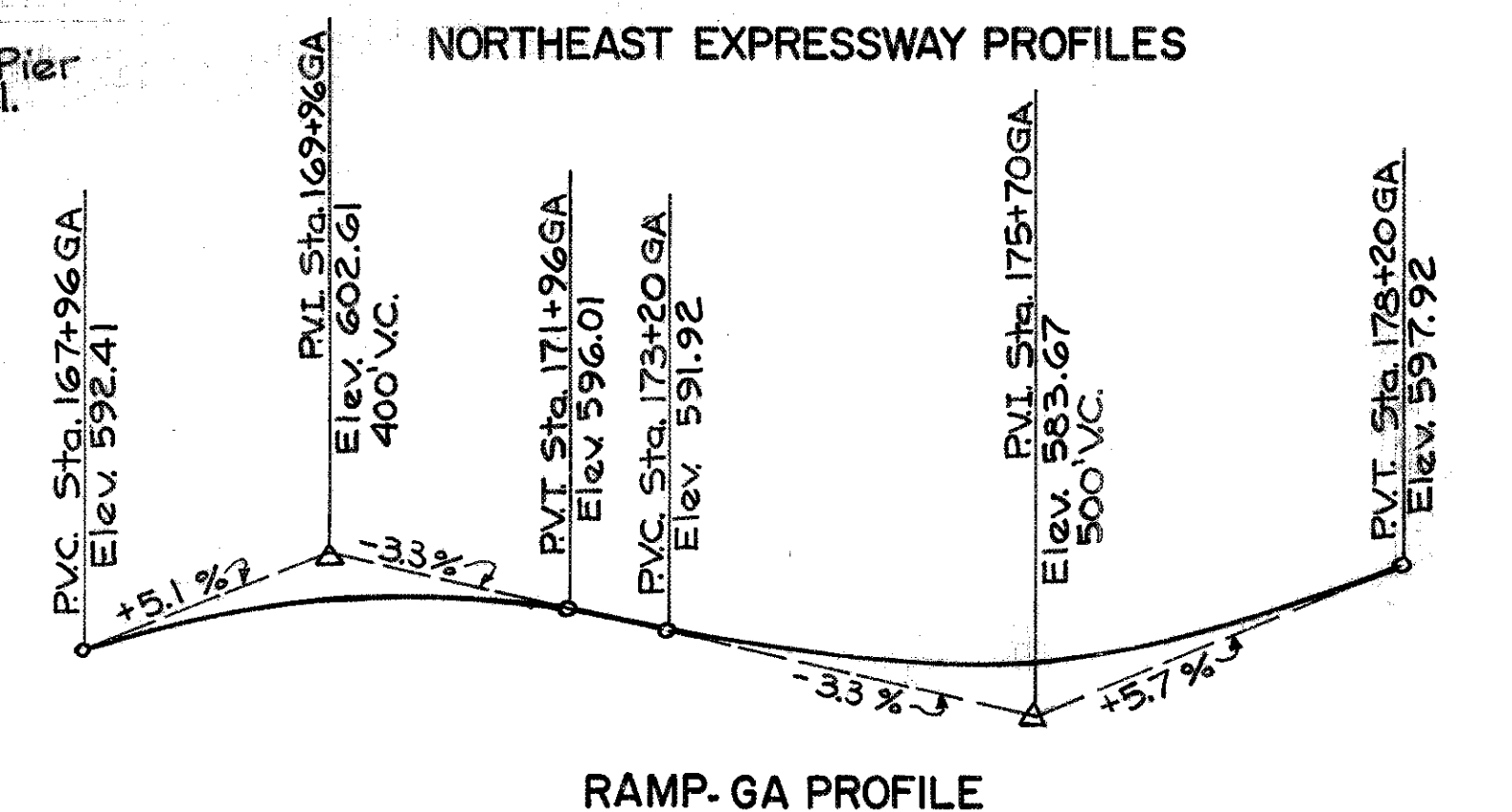
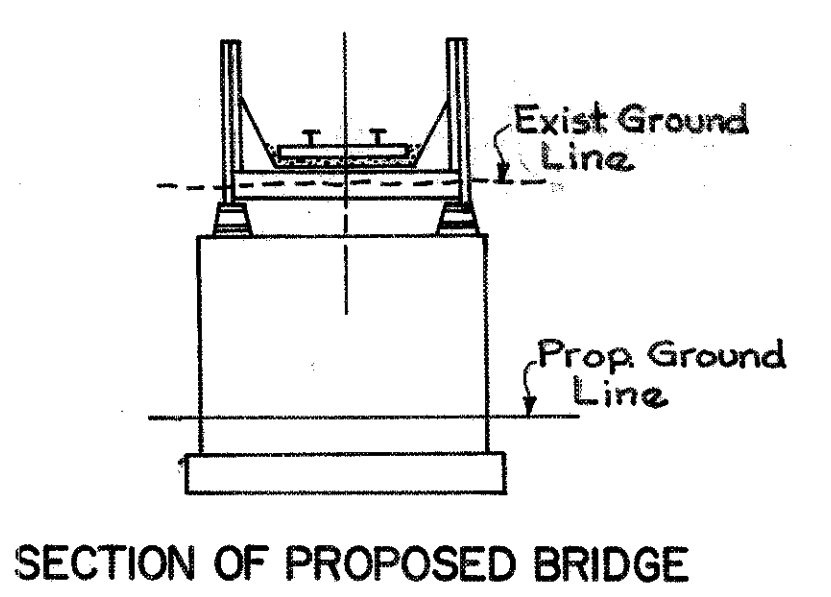
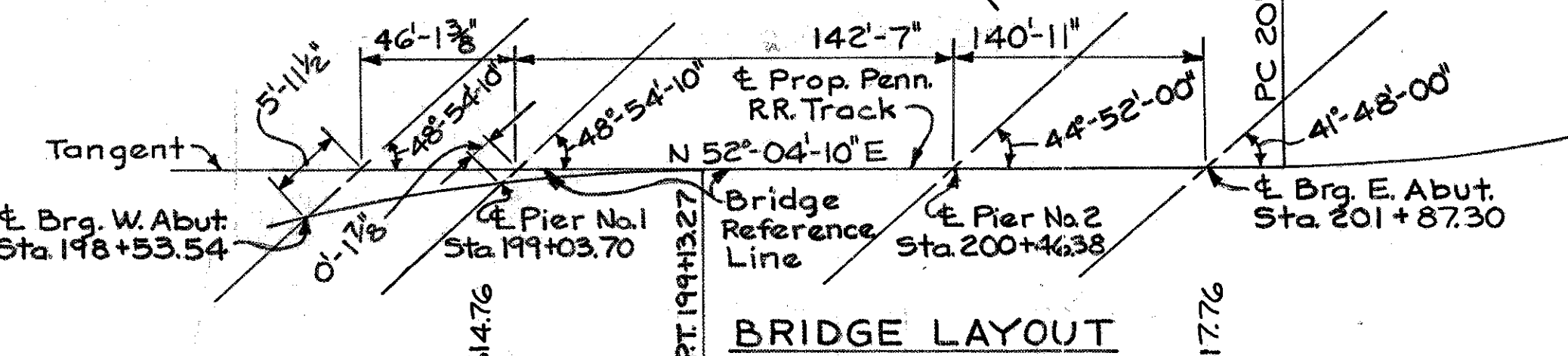
**DRAINAGE DETAILS
BRIDGE NO. HAM-71-0203**

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	C.B.H.		D.W.	J.Ho 5/20/65	

HAMILTON COUNTY
HAM-71-(1.56)(2.51)



CURVE No 2 RR DATA	CURVE No 3 RR DATA
PI. Sta 197+95.94	PI. Sta 204+22.27
$\Delta = 35^{\circ}17'00''$	$\Delta = 41^{\circ}23'26''$
$D = 14^{\circ}30'$	$D = 9^{\circ}30'$
$R = 396.20'$	$R = 603.80'$
$L = 243.33'$	$L = 435.69'$
$T = 126.00'$	$T = 228.10'$



GENERAL NOTES

- Symbol denotes drill hole. See Sheet No. 413 for test boring data.
- For Bench Marks, see Sheet No. 40
- Span lengths are based on \pm Brg. West Abut to \pm Pier No. 1 to \pm Pier No. 2 to \pm Brg. - East Abutment along Bridge Reference Line.

PROPOSED STRUCTURE

Type: Ballasted Deck Steel thru Girder Superstructure.

Span 1 is simple Girder. Spans 2 & 3 are Continuous Girders. Substructure is reinforced concrete.

Spans: 46'-1 3/8", 142'-7" & 140'-11" (See General Notes)

Live Load: Cooper E-72 with Diesel Impact

Skew: Varies, see plan.

Design Specifications: Current A.R.A.

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SITE PLAN

BRIDGE No. HAM-71-0207
UNDER PENN. R.R.

H & E BRIDGE No. 13

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	C.T.M.		H.B.	J.H.D. 5/22/65	

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	SUPER-STRUCTURE	WEST ABUTMENT	EAST ABUTMENT	PIER 1	PIER 2	GENERAL
E-2	Lump	Sum	Cofferdams, Cribs & Sheeting						Lump
E-2	902	Cu. Yds.	Unclassified Excavation		24	700	1/8	160	
S-1	287	Cu. Yds.	Class C Concrete, Piers above Footings				160.4	126.6	
S-1	205	Cu. Yds.	Class E Concrete, Abutments above Footings		106.0	99.0			
S-1	363	Cu. Yds.	Class E Concrete, Footings		103.6	98.7	74.7	86.0	
S-3	220	Sq. Yds.	Type A Waterproofing		112	108			
S-3	41	Lin. Ft.	Waterproofing, Premolded Sealing Strip		20	21			
S-4	61681	Lbs.	Reinforcing Steel		25323	22174	6321	7863	
S-107	158000	Lbs.	Structural Steel - Wrought Iron (Deck)	158000					
S-107	895000	Lbs.	Structural Steel	995000					
S-8	1153000	Lbs.	Field Painting of Structural Steel	1,153,000					
S-14	13	Lin. Ft.	Railing, Type C (Aluminum Rail, Supports, Concrete Parapet & End Posts)		4	9			
S-16	Lump	Sum	First Test Pile						Lump
S-18	8239	Lin. Ft.	Steel Piles, 10BP42		1994	1789	1864	2592	
S-29	124	Cu. Yds.	Porous Backfill		65	59			
S-29	209	Lin. Ft.	6" Standard Pipe Downspout, Wrought Iron, or Hot-dipped Galvanized Steel, Including Specials		51		82	76	
S-29	24	Lin. Ft.	8" Bituminous Coated Helical Perforated Corrugated Metal Pipe (Sec. M6, 4(h-c) (Including Specials)		12	12			
S-29	86	Lin. Ft.	8" Bituminous Coated Helical Perforated Corrugated Metal Pipe (Sec. M6, 4(h-c) (Including Specials & Sand)		44	42			
S-29	9	Lin. Ft.	12" Reinforced Concrete Sewer Pipe Sec. M-6, 6(a)		9				
S-29	677	Lin. Ft.	8" W. I. Half Round Deck Drain with Specials	677					
Special	36	Cu. Yds.	Asphalt Portland Cement Concrete	36					
Special	170	Lin. Ft.	Prebored Holes		170				

GENERAL NOTES

DESIGN SPECIFICATIONS: The design of this structure conforms to the requirements of the Specifications for Steel Railway Bridges of the American Railway Engineering Association, 1934 Edition, except that sections governed by a loaded length of live load of 60 feet or less shall be based on the unit stresses given in the 1961 Edition for ASTM A-7 steel.

Live Load - Cooper's E72.

Impact for diesels without hammer blow.

Concrete Class C - basic unit stress 1,333 psi
Concrete Class E - basic unit stress 1,133 psi

Structural Steel - ASTM A36 basic unit stress 20,000 psi except as noted above. (ASTM A7 or A373 steel not permitted except for piling.) Plates 3/4" to 1-1/2" in thickness that carry stresses shall be silicon killed fine grain practice.

Reinforcing Steel - ASTM A15, A16, A160. Deformed Intermediate or Hard Grade. Basic unit stress 20,000 psi.

CONSTRUCTION SPECIFICATIONS: State of Ohio, Department of Highways, Construction and Material Specifications, 1963, also Supplemental Specification No. S-107 revised February 16, 1955, Supplemental Specification No. S-307 revised 10-1-64 and Standard Drawing AR-1-57 revised 4-2-62, shall govern this project; except as otherwise noted on the plans.

WELDING to be in accordance with American Welding Society Specifications for Welded Highway and Railway Bridges D2, 0-63. All welds to be continuous unless otherwise shown. All welders to be qualified in accordance with P. R. R. regulations or other regulations meeting with the approval of the Engineer.

All welds connecting flange plate to web plate must be made by automatic submerged arc welding.

All grinding shall be done in the direction of applied stresses. Transverse tack welds will not be permitted on the tension flange plates.

50% of web flange weld to be subjected to magnetic particle inspection.

Mill scale to be ground off flange plates at web to flange weld.

PILES shall be driven to firm contact with rock. If the length of penetration is approximately equal to the depth to rock according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity, according to the formula in Sec. S-18.05, is not less than 45 tons per pile using a 15,000 ft. lb. hammer.

The design of abutments and piers is based on 35 tons per pile.

TIES, RAILS, FASTENINGS and other railroad equipment will be furnished and installed by the Railroad Company.

DOWNSPOUTS shall be 6" standard wrought iron pipe or hot-dipped galvanized steel pipe. Joints shall be made by welding or by the use of a clamp type coupling with a ring gasket. All welding shall be done before galvanizing. Straps or clamps for attaching downspouts shall be wrought iron or hot-dipped galvanized steel. On bolts, galvanizing as called for in Sec. M-10.30 will be considered sufficient.

EXCAVATION: For the purpose of driving battered piles at the East Abutment (shown hatched on Sheet No. 417) shall be included with "Unclassified Excavation" for payment.

DECK WATERTIGHTNESS: The deck shall be watertight.

TYPE A WATERPROOFING shall be applied to the back of the abutments from top of footings to top of backwall and to finished ground line.

ADDITIONAL NOTES: For additional notes see Notes 2, 6 & 8 General Notes, Typical Drawing No. 443.

ITEM SPECIAL - ASPHALT PORTLAND CEMENT CONCRETE

Use:
This concrete is to be used as a protection course over membrane waterproofing and as a protection course over metal decks of railroad bridges.

Materials:

1. **Emulsified Asphalt** shall be in accordance with Federal Specifications SS-A-674b.

2. **Fine aggregate** shall be as specified for concrete.

3. **Coarse aggregate** shall be as specified for concrete with the following gradation:

Sieve Size (Inches)	Percent Passing by Weight
3/4"	100
1/2"	90 - 100
3/8"	40 - 70
No. 4	0 - 15

Mix:

The asphalt concrete shall be proportioned by volume, 1 part portland cement, 2 parts fine aggregate, 3 parts coarse aggregate, 1-1/2 parts emulsified asphalt Type SS-1 and minimum amount of water required to make a workable mix. The dry materials shall first be thoroughly mixed then the water added to produce a uniform mixture after which the emulsified asphalt is added and mixing continued until of uniform color.

Application:

Work shall be done only when the temperature will be above 40°F. for a period of at least 24 hours after starting application.

Surface to be covered shall be thoroughly cleaned and given a uniform primer or bond coating of Emulsified Asphalt Type RS-1, using not less than 1 gallon per 80 sq. ft.

The asphalt concrete shall then be screeded over the primed area to the thickness indicated on the drawings. After the asphalt concrete has set sufficiently to allow a man to walk upon it without indentation but prior to final set, it shall be given a uniform coating of emulsified asphalt type RS-2 applied at the rate of one gallon per 50 sq. ft. This final coating shall be covered with clean sand.

METHOD OF MEASUREMENT:

The yardage to be paid for shall be the number of cubic yards as determined by calculations from plan dimension, in place, completed and accepted.

BASIS OF PAYMENT:

The yardage measured as provided above shall be paid for at the contract unit price per cubic yard bid for "Item Special Asphalt Portland Cement Concrete" which price and payment shall constitute full compensation for furnishing, placing, finishing and curing, and all labor, equipment, tools and incidentals necessary to complete the item.

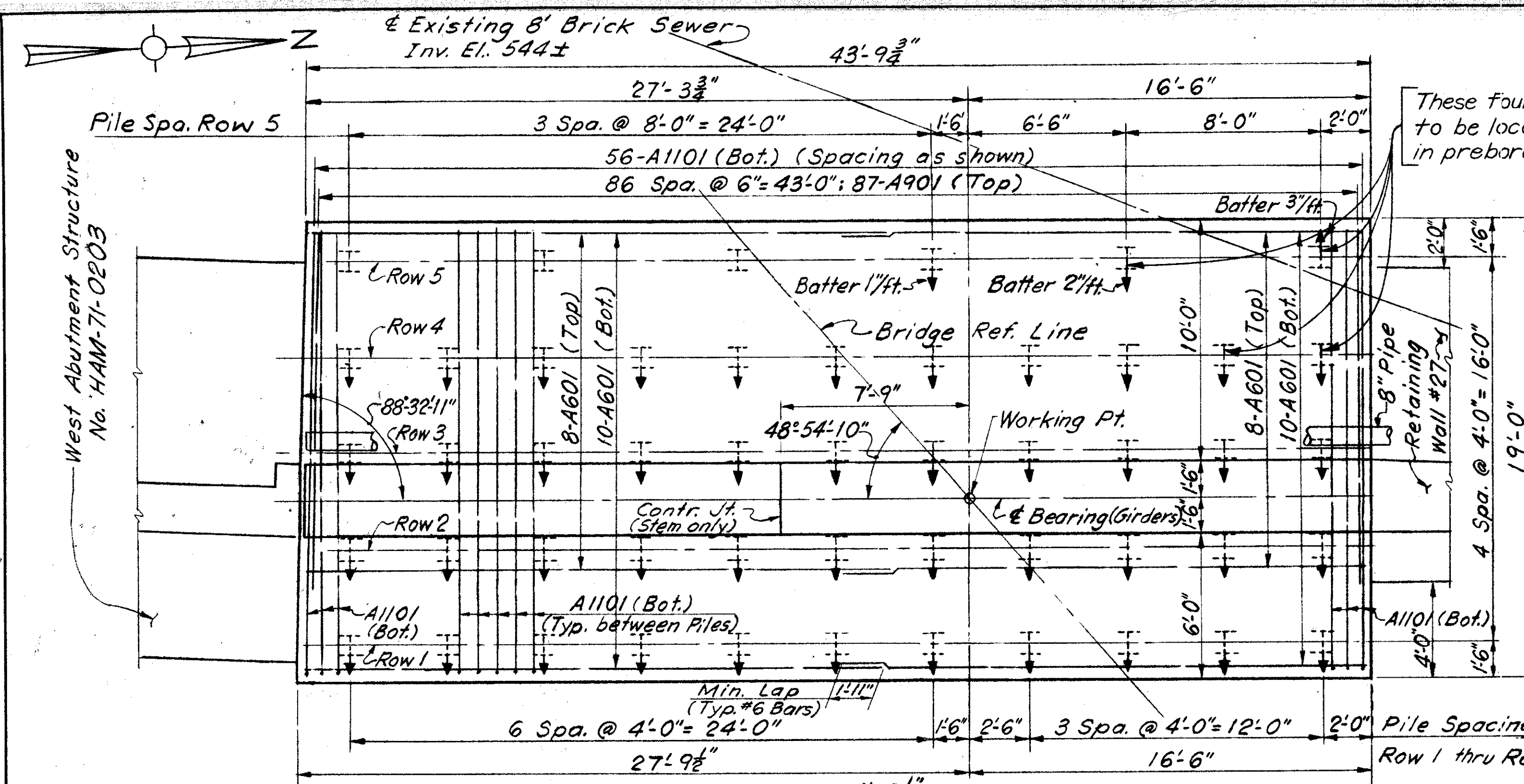
PREBORED HOLES: Where directed by the Engineer, piles of West Abutment as indicated on the footing plan Sheet No. 419 adjacent to the existing 8" Brick Sewer, shall be started in 12" minimum diameter prebored holes to a minimum depth of 1'-0" below the bottom of sewer. Payment shall include the preboring of the hole and also filling of the hole with materials conforming to Item M-2.1, in the contract unit price per linear foot bid for "Item Special".
The Contractor shall verify the line and grade of the existing 8" Brick Sewer before starting any prebored holes.

HAMILTON COUNTY CONSULTING ENGINEERS CINCINNATI, OHIO					
ESTIMATED QUANTITIES AND GENERAL NOTES BRIDGE NO. HAM-71-0207					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVIEWED
	BJM		ME	JHO 5/22/65	

SHEETING AND BRACING: Before construction is started, eight sets of prints showing details of the sheeting and bracing to be used for excavation adjacent to the railroad tracks shall be submitted to the Director for approval by the Department of Highways and by the Railroad Company.

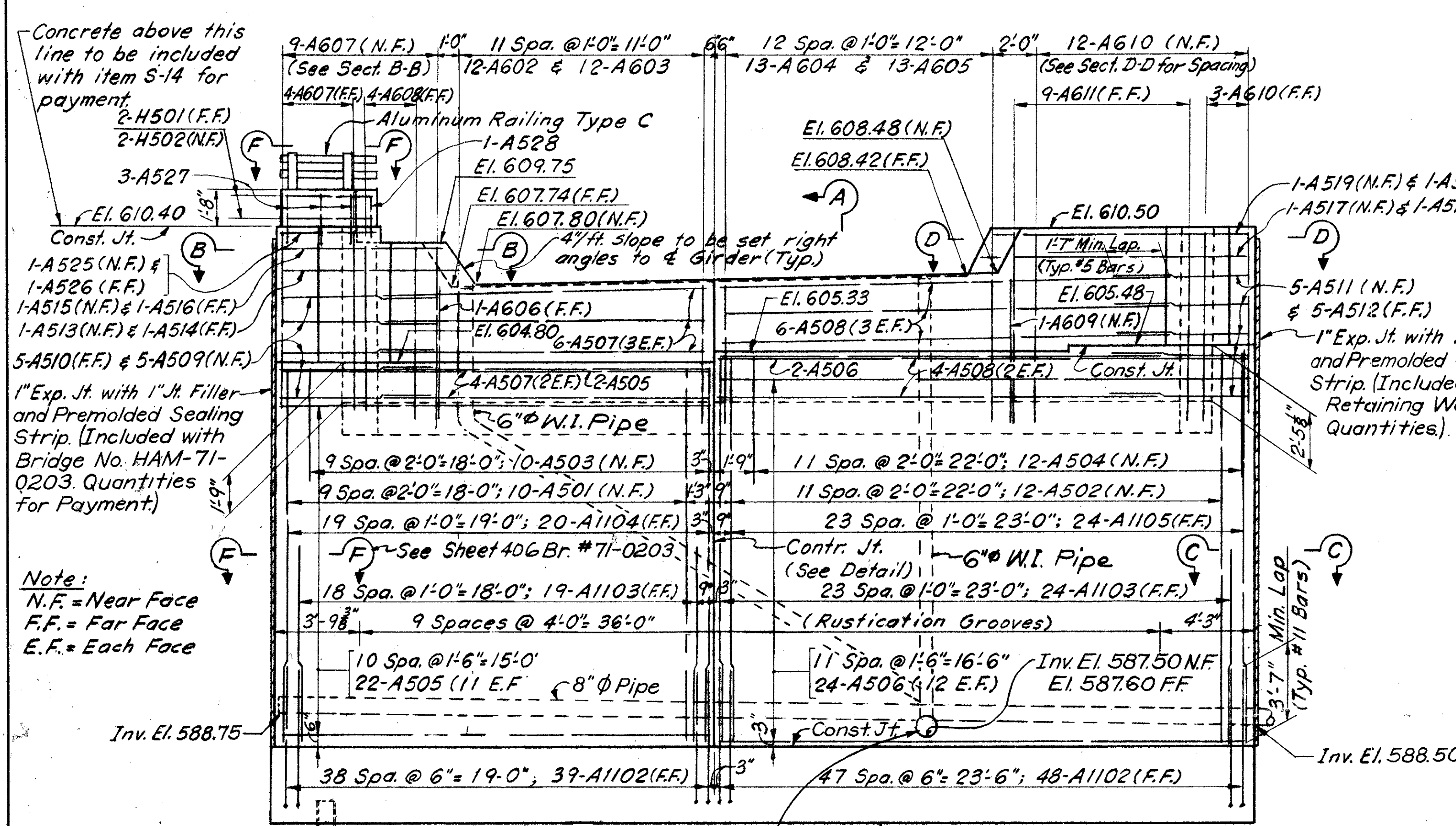
EXCAVATION OF UNSUITABLE MATERIAL: After the unsuitable materials have been removed to elevation 580.0, at the West Abutment as shown on the Site Plan, the Engineer shall make a careful inspection to ascertain that all unsuitable materials have been removed to a stable subsoil. If necessary, the excavation limits shall be extended to remove additional unsuitable materials and be replaced with materials meeting the requirements of Section E-1 of the Specifications.

Embankment to replace unsuitable material shall be compacted to a minimum of 8" above the bottom of footing elevations before proceeding with structure excavation, (Item E-2, unclassified excavation) at the West Abutment and Pier 1.



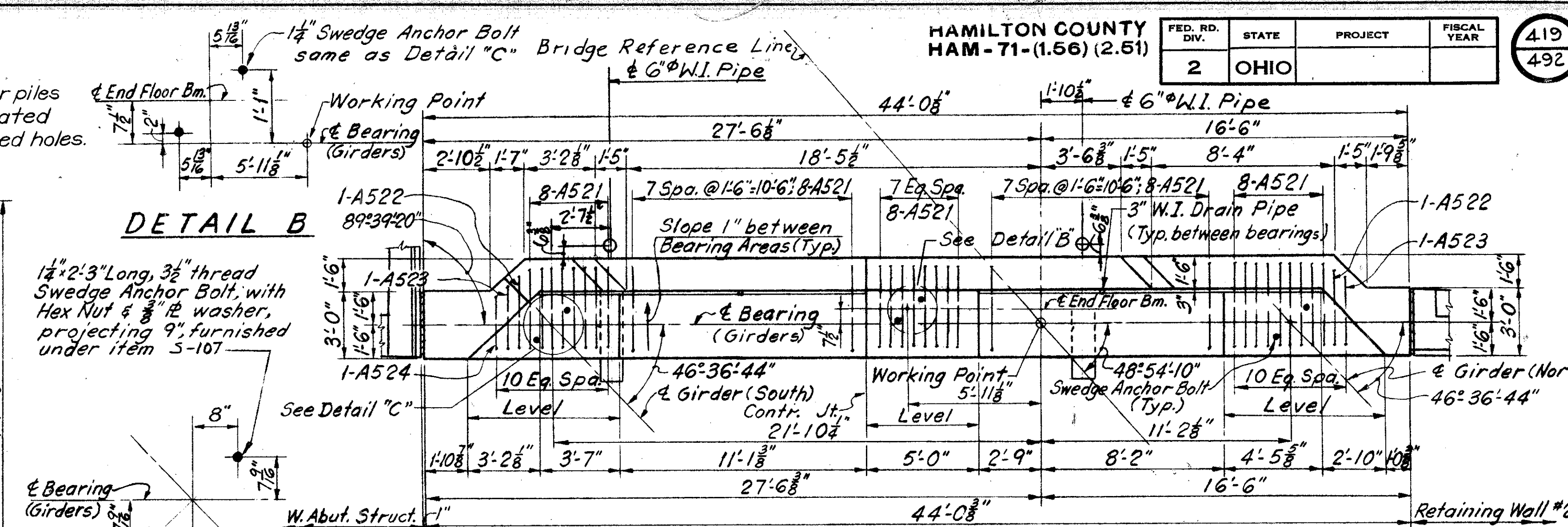
FOOTING PLAN

Note: Care shall be taken in driving piles in the vicinity of the existing brick sewer to avoid any damage to it.

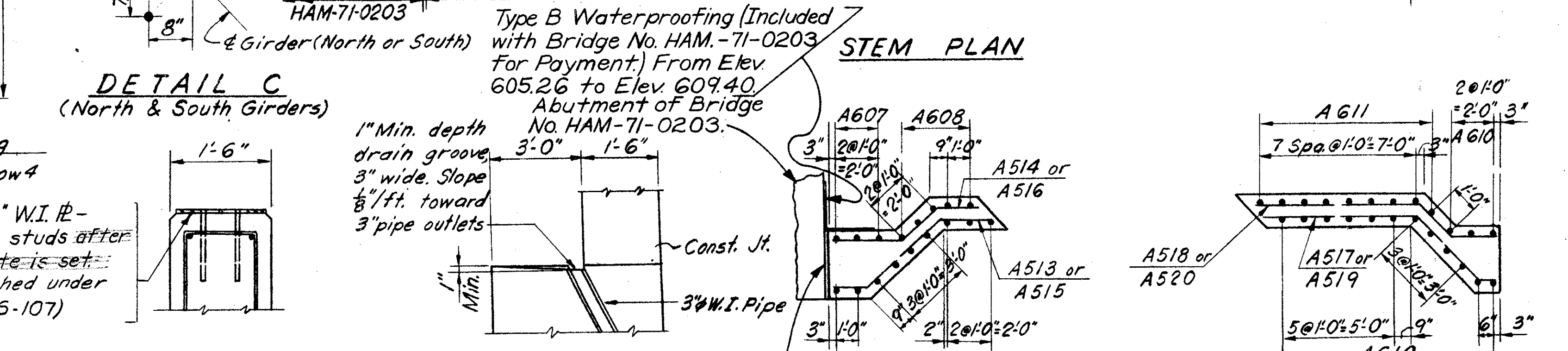


FRONT ELEVATION

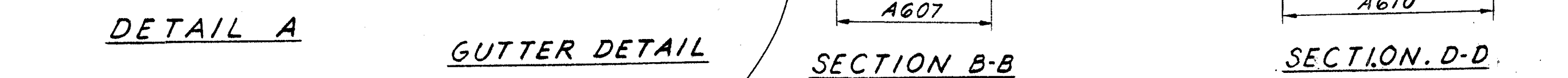
Note:
N.F. = Near Face
F.F. = Far Face
E.F. = Each Face



DETAIL B

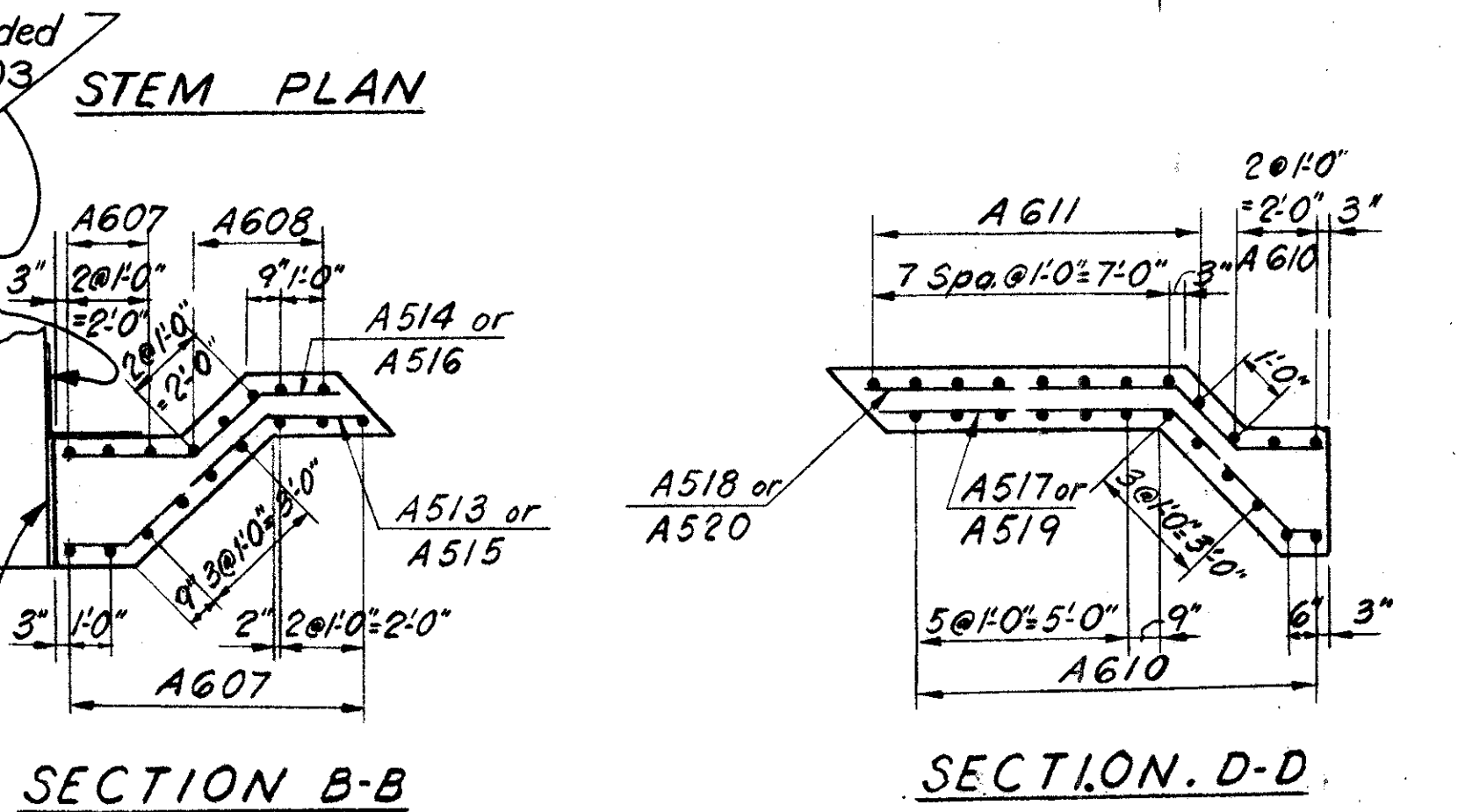


DETAIL C
(North & South Girders)



DETAIL A

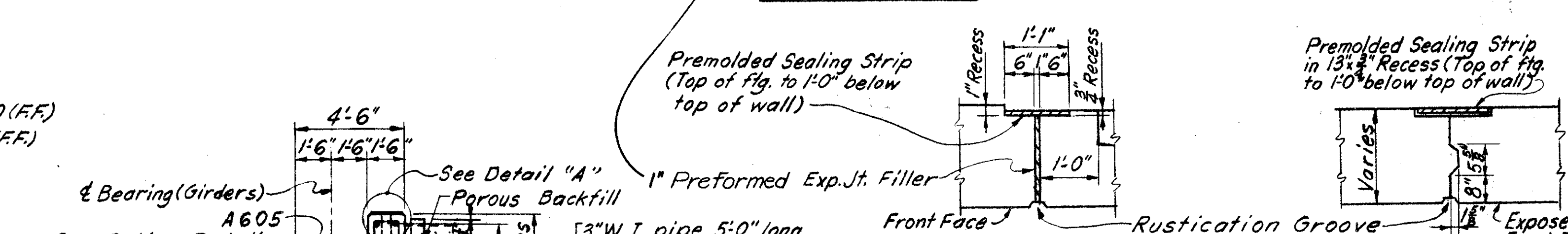
GUTTER DETAIL



STEM PLAN

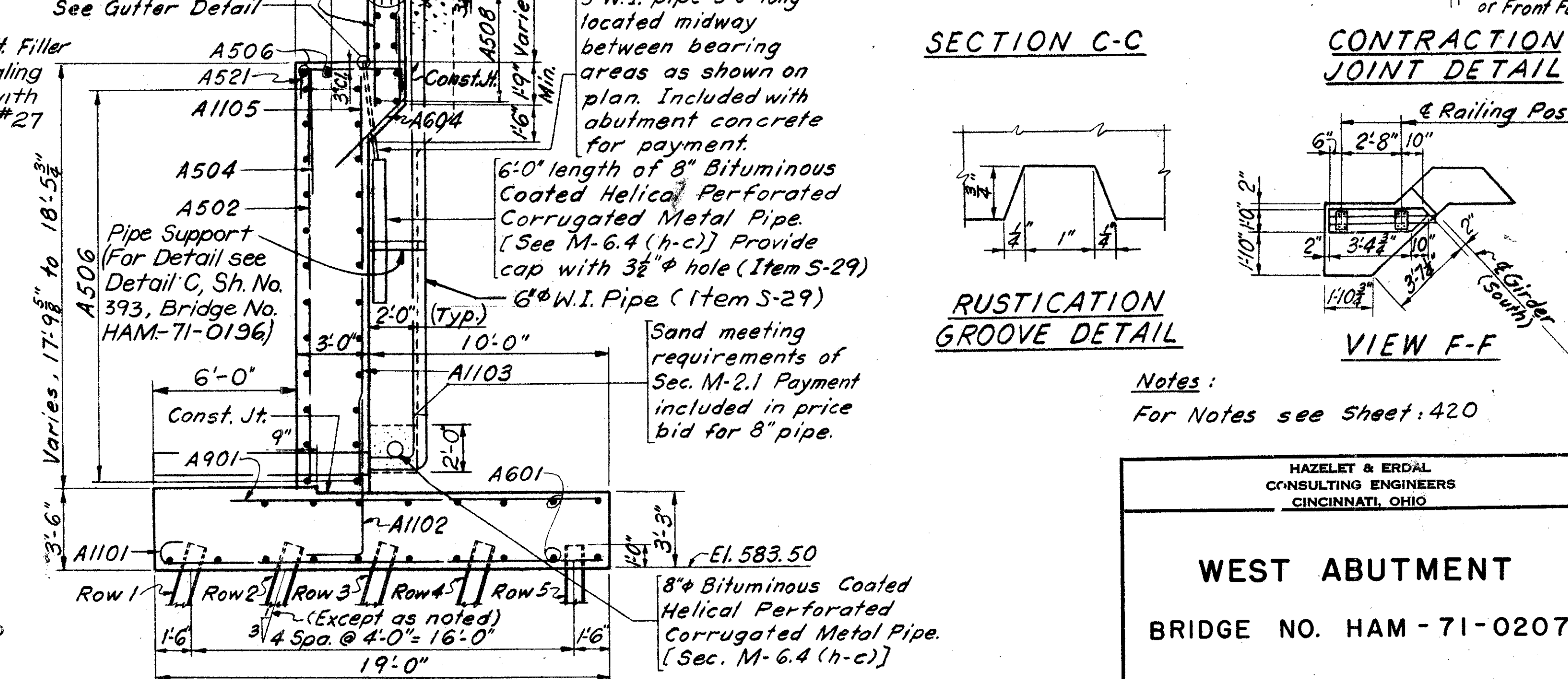
SECTION B-B

SECTION D-D



SECTION C-C

CONTRACTION JOINT DETAIL



SECTION A-A

RUSTICATION GROOVE DETAIL

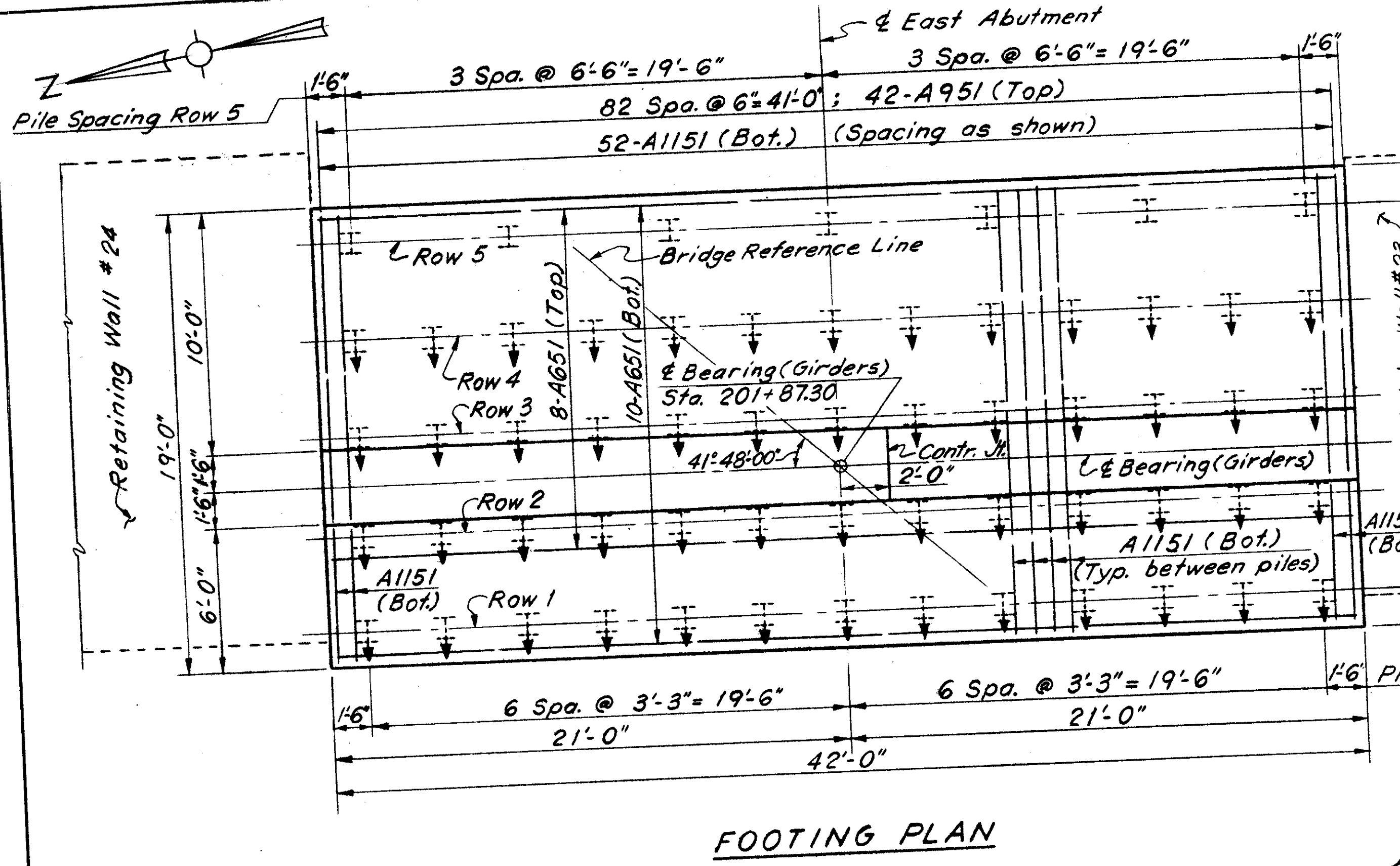
VIEW F-F

Notes:
For Notes see Sheet 420

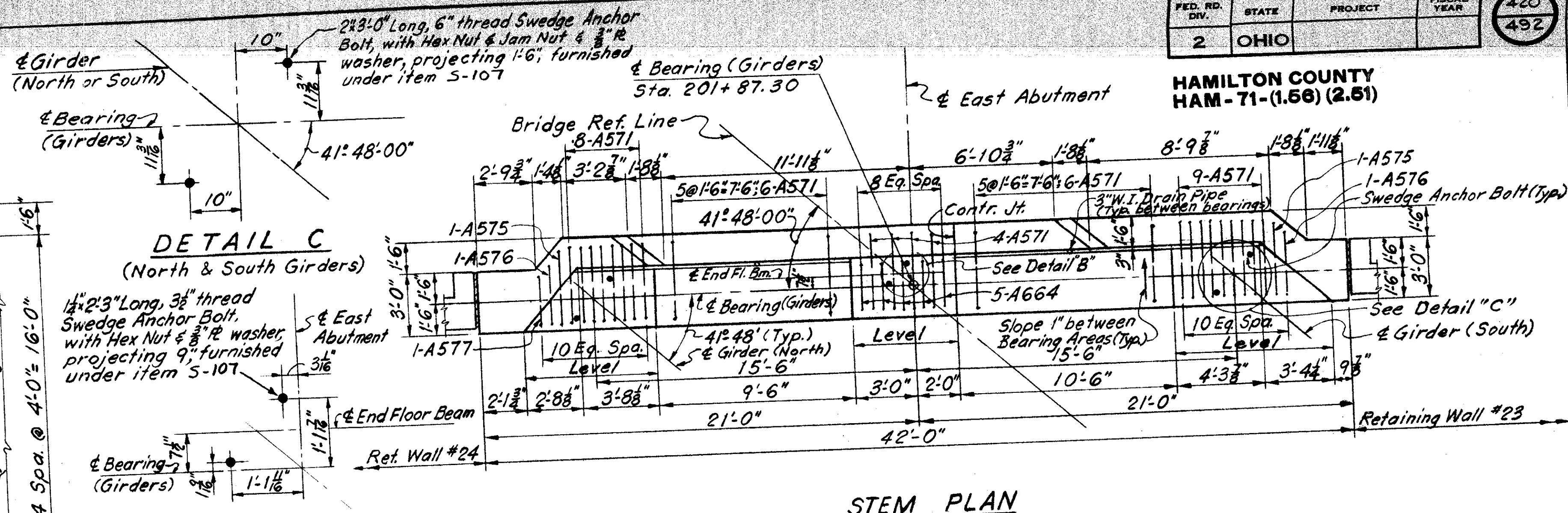
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
WEST ABUTMENT				
BRIDGE NO. HAM-71-0207				
DESIGNED TFL	DRAWN TFL	TRACED	CHECKED D.W.	REVIEWED DATE JHO 5/22/65

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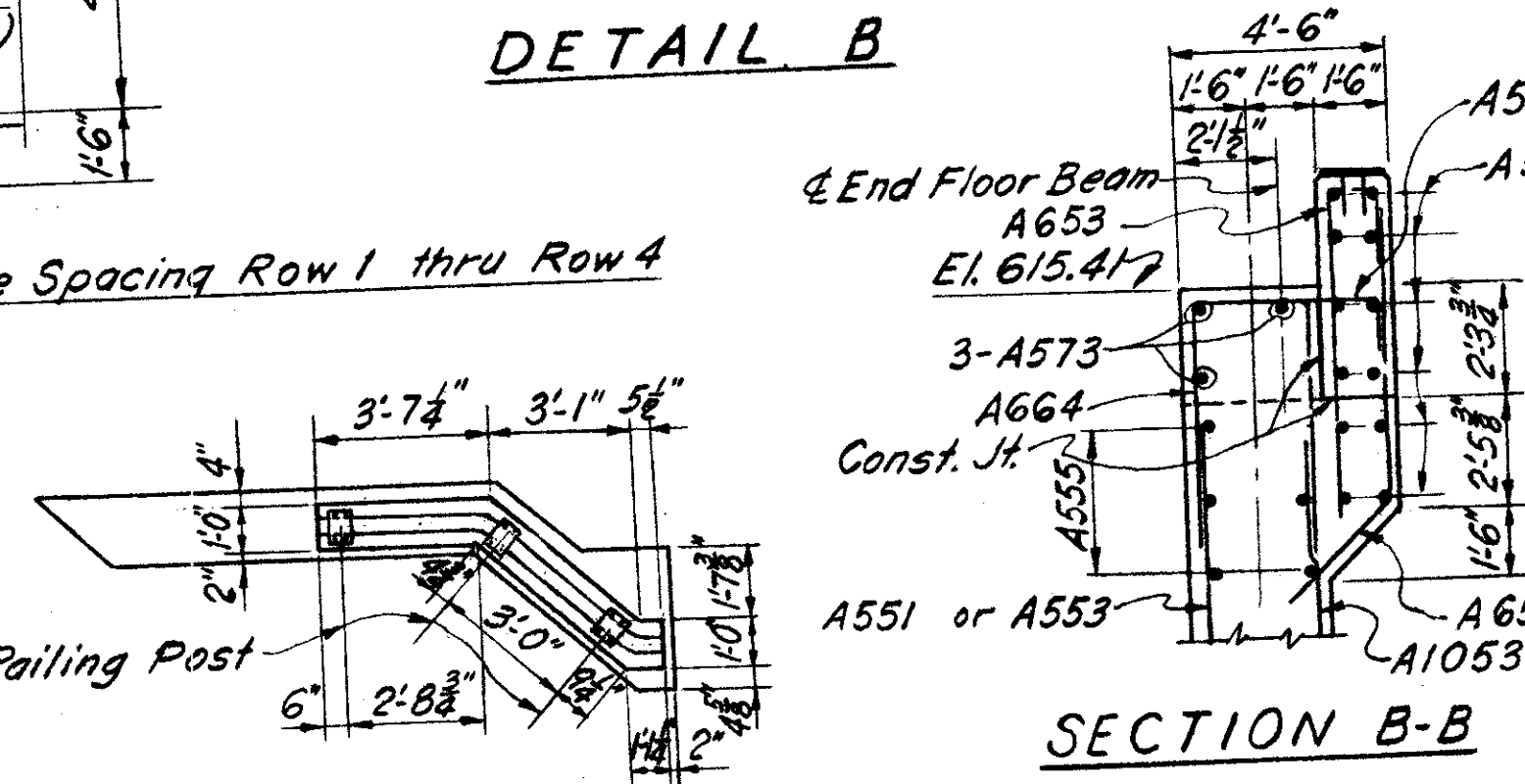
HAMILTON COUNTY
HAM - 71 - (1.58) (2.51)



FOOTING PLAN



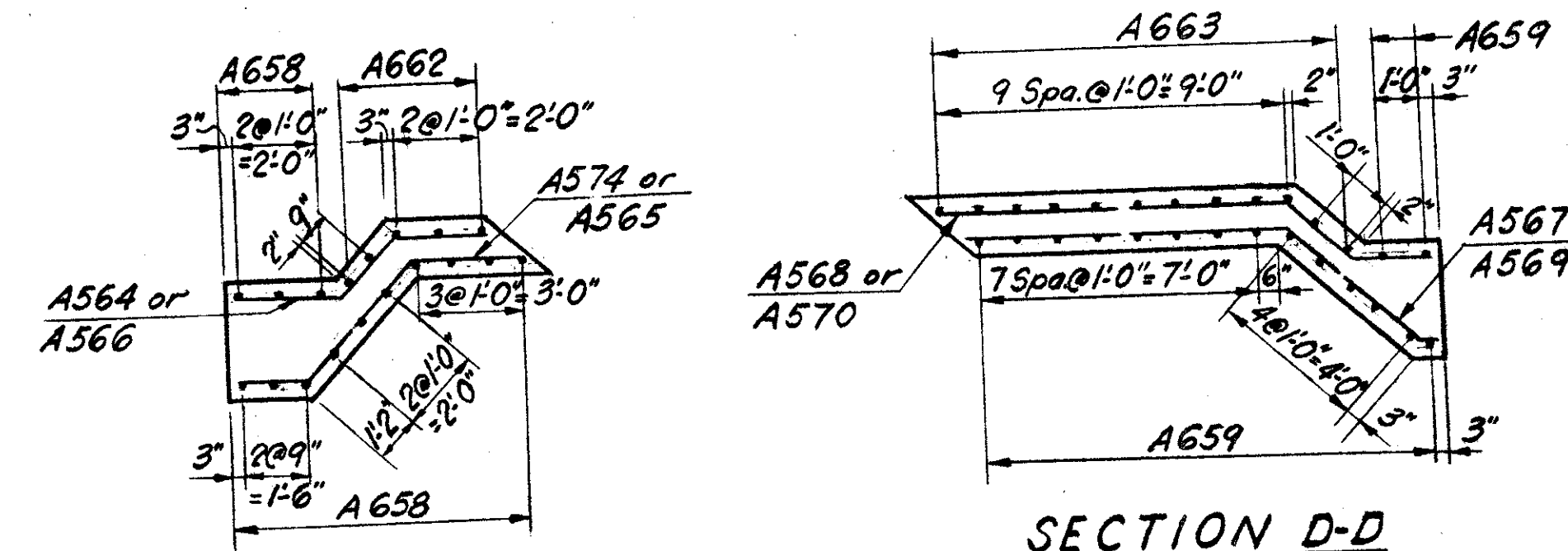
DETAIL C
(North & South Girders)



DETAIL B

VIEW G-G

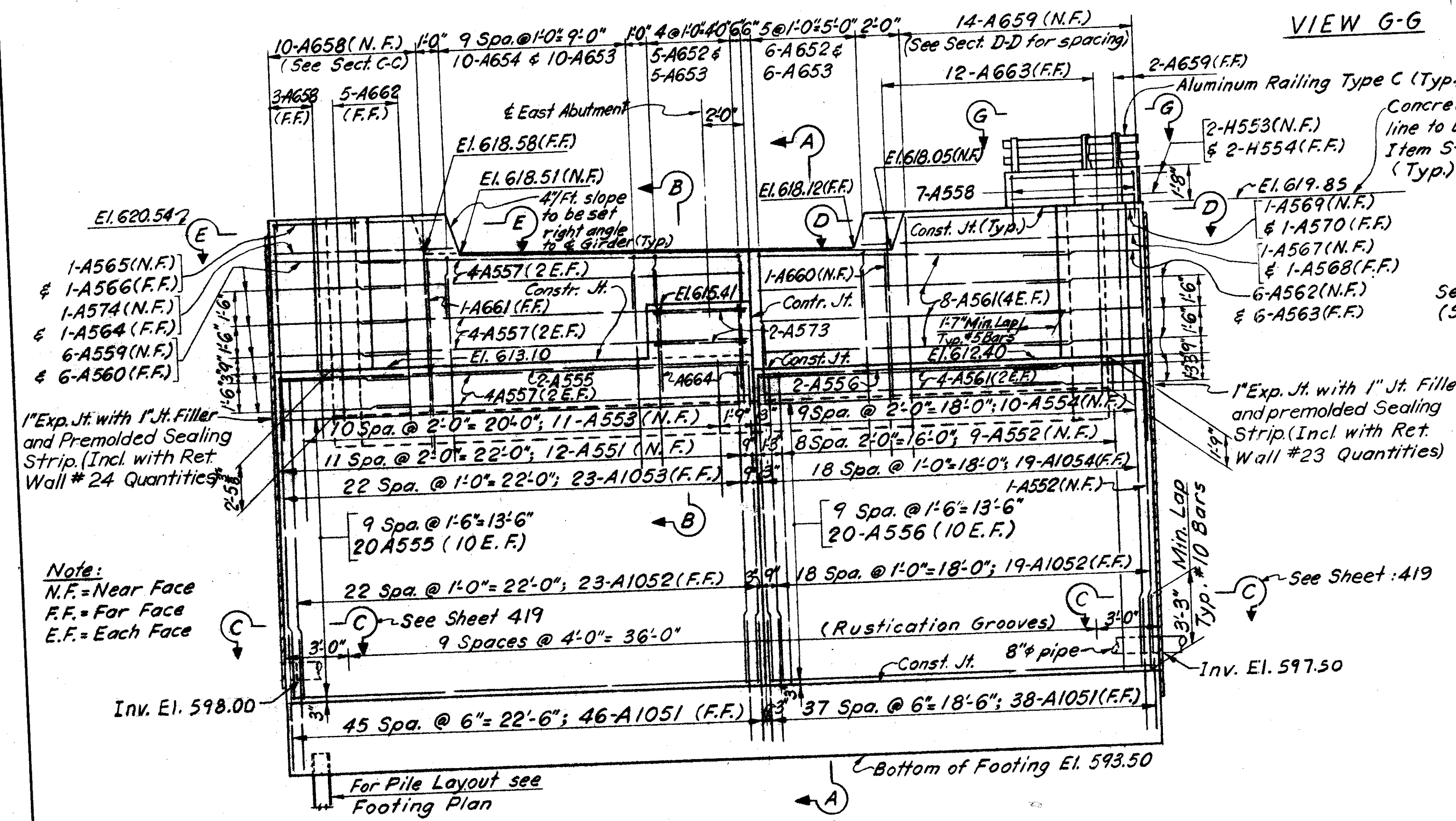
STEM PLAN



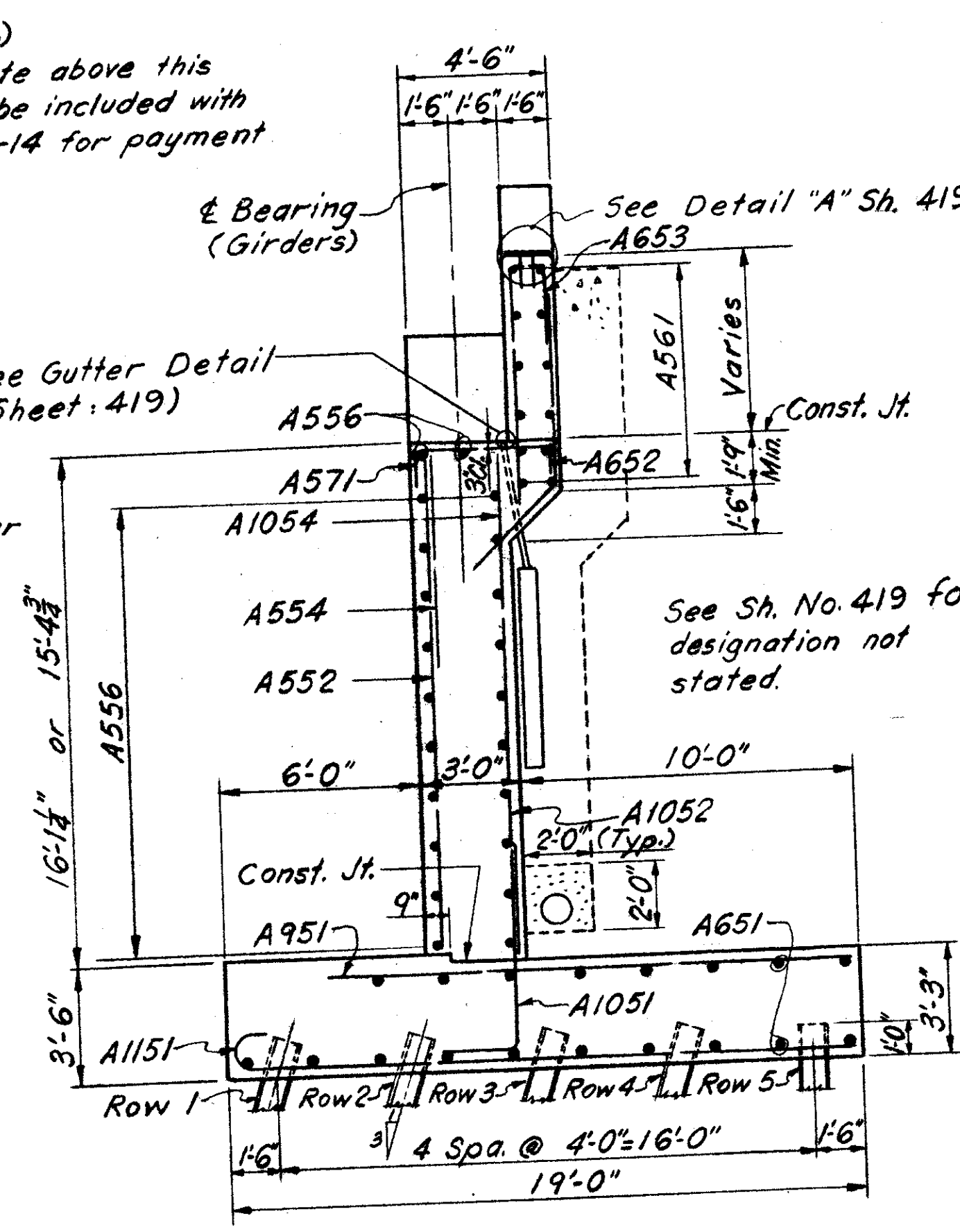
SECTION E-E

SECTION B-B

SECTION D-D



FRONT ELEVATION



SECTION A-A

Notes:
 Provide 3" clearance to reinforcing steel in footing and 2" in stem (Min).
 Abutment concrete shall be Class "E".
 Concrete above bridge seat construction joint shall not be placed until Superstructure Steel is in place.
 Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of swedge anchor bolt holes.
 Porous Backfill 2 ft. thick, full length of abutment shall extend up to the bottom of ballast.
 All piles shall be 10 BP42 Steel H piles.
 H denotes vertical piles
 H denotes batter piles (4:12 except as noted).
 For railing details not shown, see Standard Drawing No. AR-1-57.

PROCEDURE: Before the backwall is constructed the backfill behind the West Abutment shall be placed and compacted up to the level of the existing ground (with all slope from the bridge seat to the existing ground) and extending to the limits for excavation of unsuitable material.

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CINCINNATI, OHIO

EAST ABUTMENT
BRIDGE NO. HAM - 71 - 0207

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DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
TFL	TFL		D.W.	JHO 5/22/65	

Note:
 N.F. = Near Face
 F.F. = Far Face
 E.F. = Each Face

1" Exp. Jt. with 1" Jt. Filler and Premolded Sealing Strip. (Incl. with Ret. Wall #24 Quantities)

1" Exp. Jt. with 1" Jt. Filler and premolded Sealing Strip. (Incl. with Ret. Wall #23 Quantities)

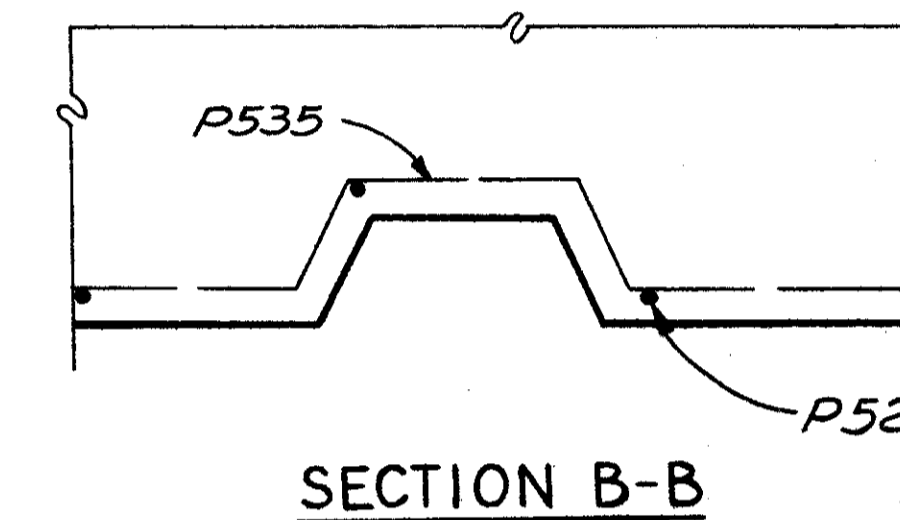
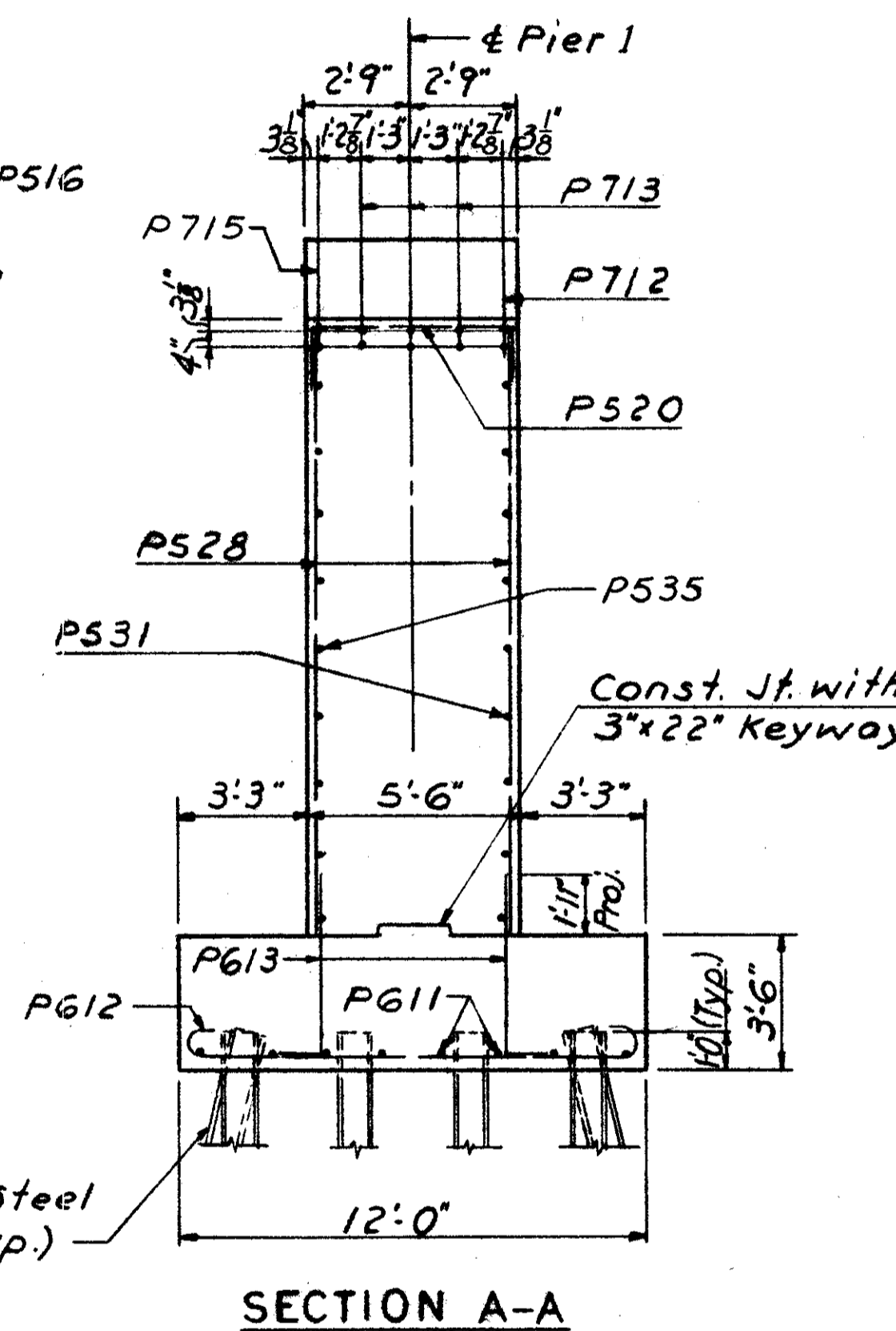
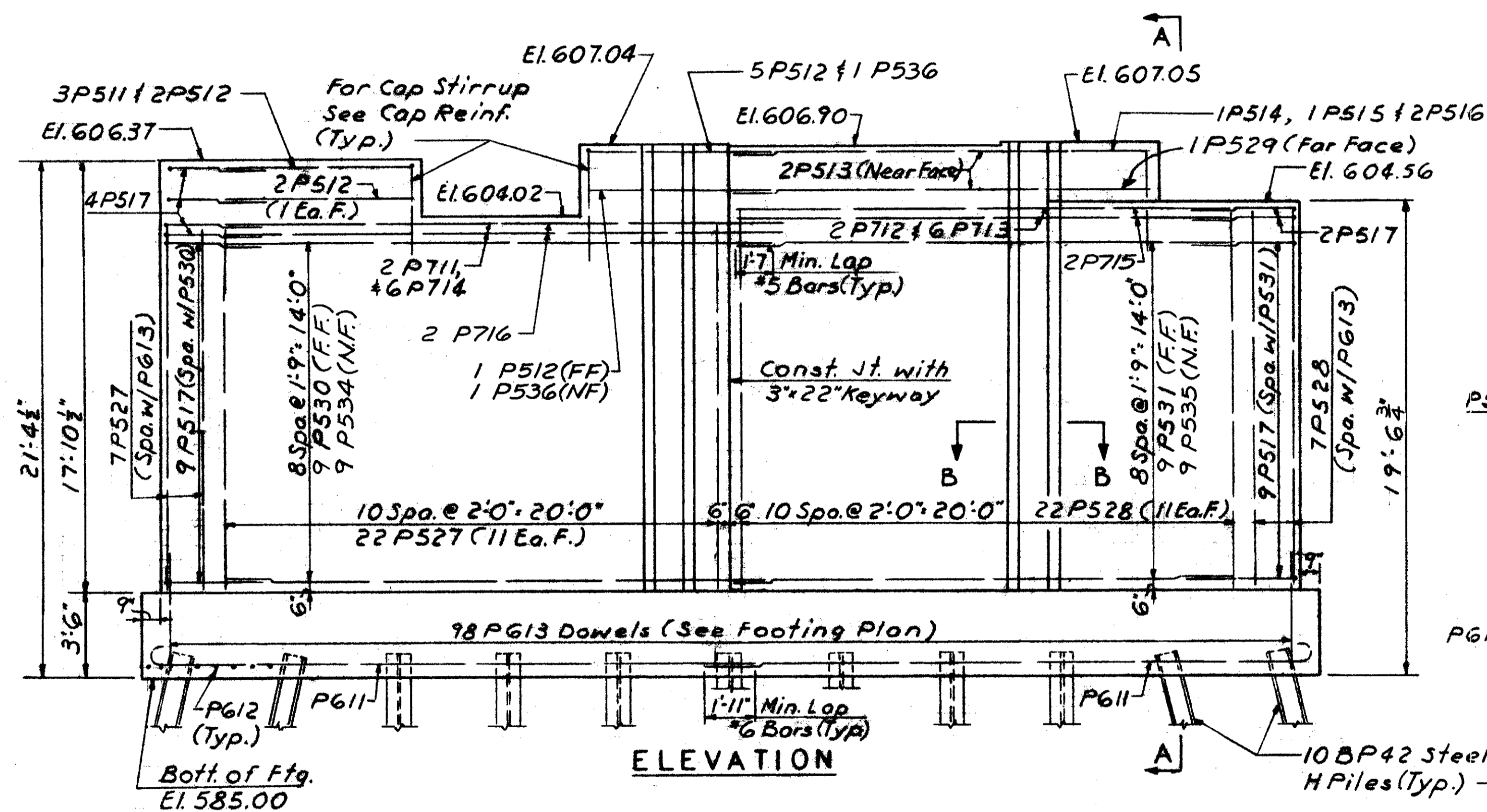
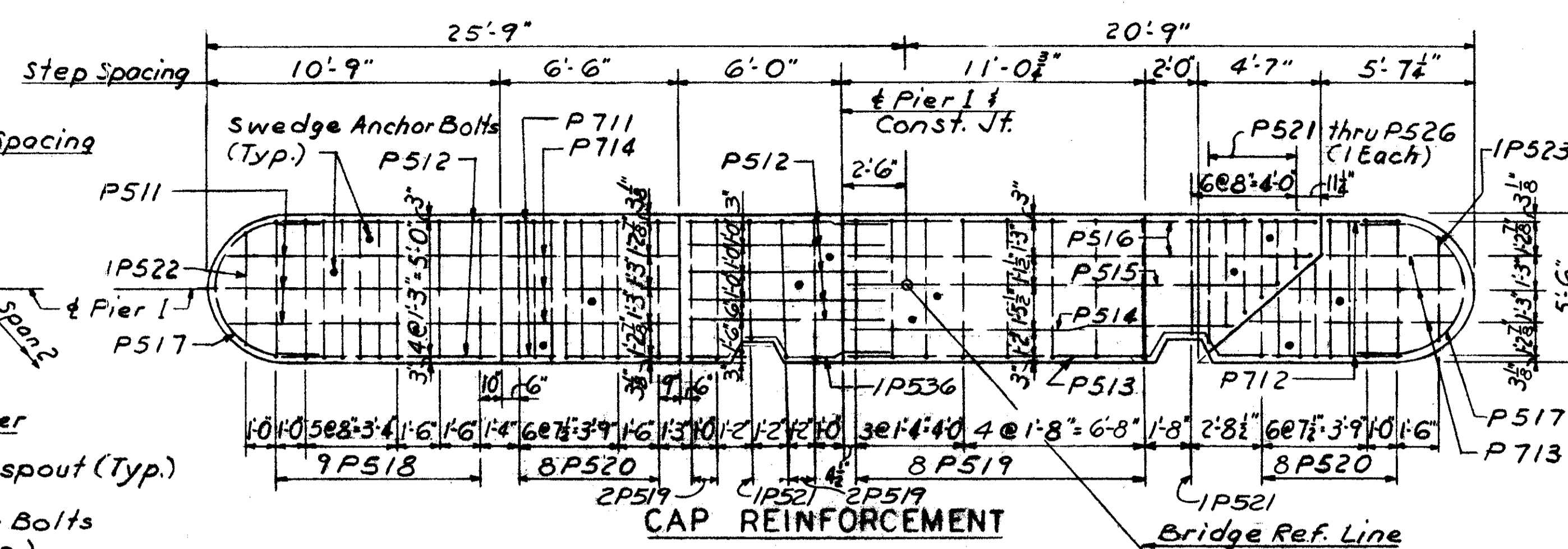
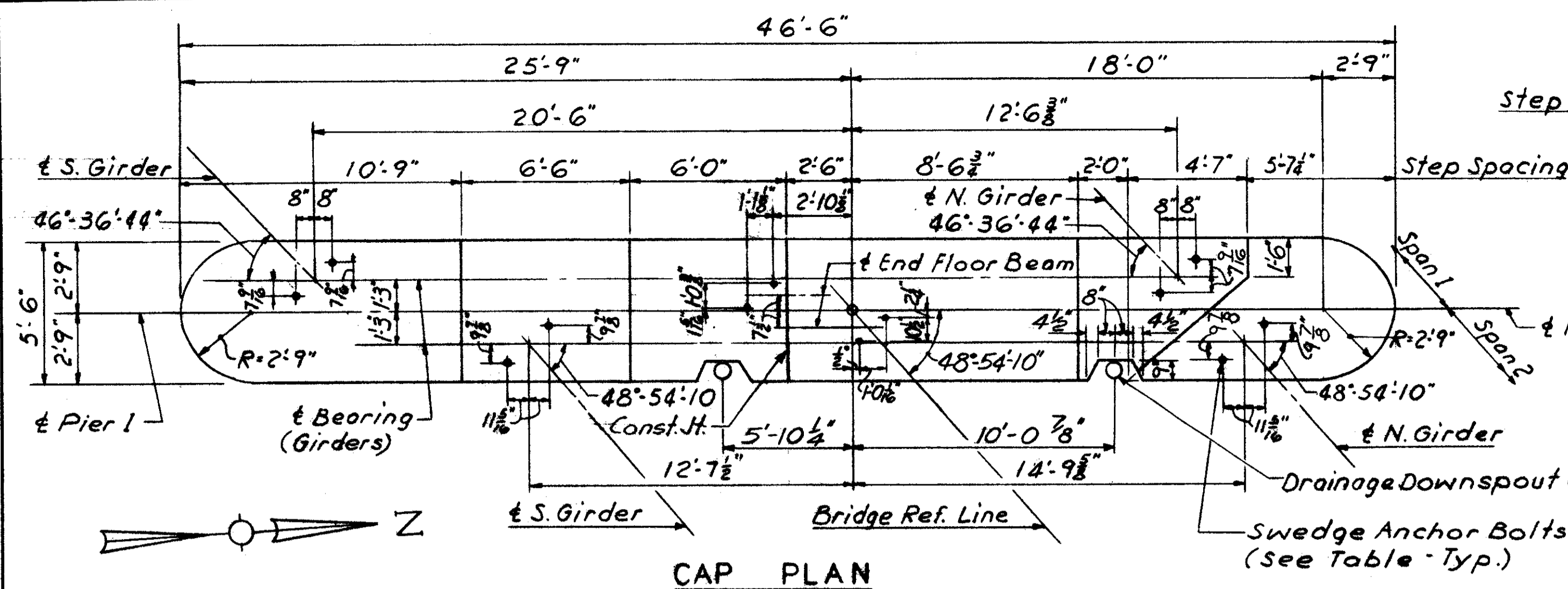
See Gutter Detail (Sheet: 419)

See Sh. No. 419 for designation not stated.

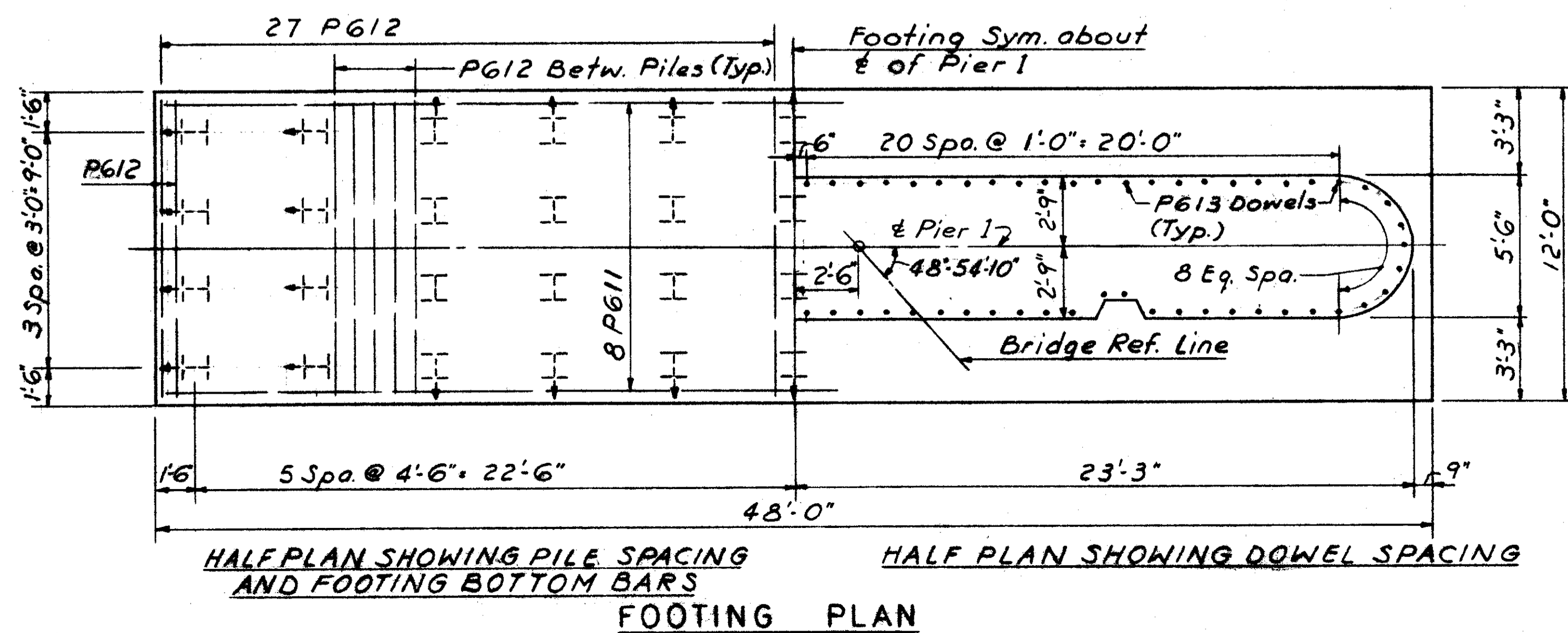
See Sheet: 419

For Pile Layout see Footing Plan

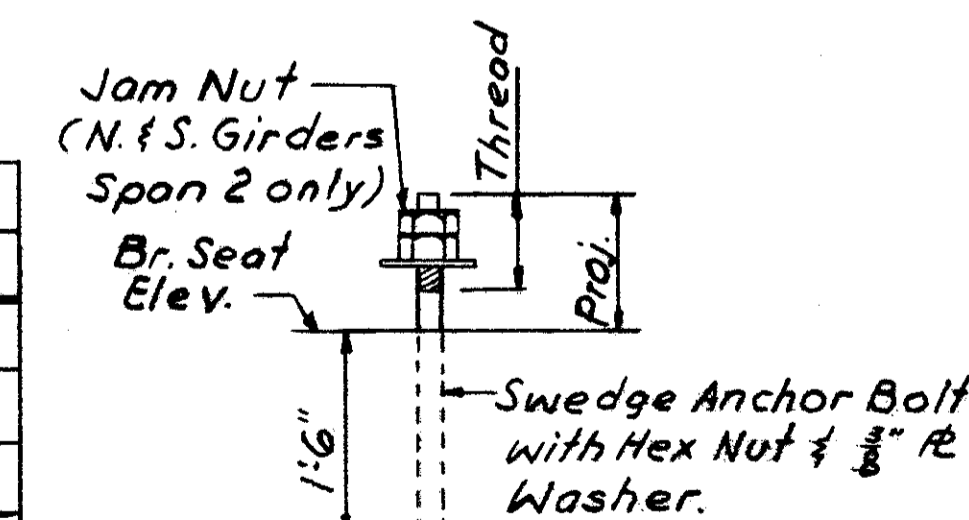
HAMILTON COUNTY
HAM-71-(1.58) (2.51)



Notes:
Provide 3" clearance to reinforcing steel in footing and 2" in wall (Min.)
Concrete in footing shall be class "E" and concrete in wall shall be class "C".
Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of swedge anchor bolt holes.
Ea.F. denotes each face.
All Piles shall be 10BP42 Steel H Piles.
H denotes Vertical Piles.
+ denotes Batter (4:12) Piles.
For Vertical Construction Joint Details see Sheet: 422



SWEDGE ANCHOR BOLT TABLE				
Location	Size & Length	Thread	Proj.	
Span 1	N & S Girder	1 1/2" x 2'-0"	3 1/2"	6"
	End Floor Bm	1 1/2" x 2'-0"	3 1/2"	6"
Span 2	N & S Girder	2" x 3'-0"	6"	1'-6"
	End Floor Bm	1 1/2" x 2'-3"	3 1/2"	9"



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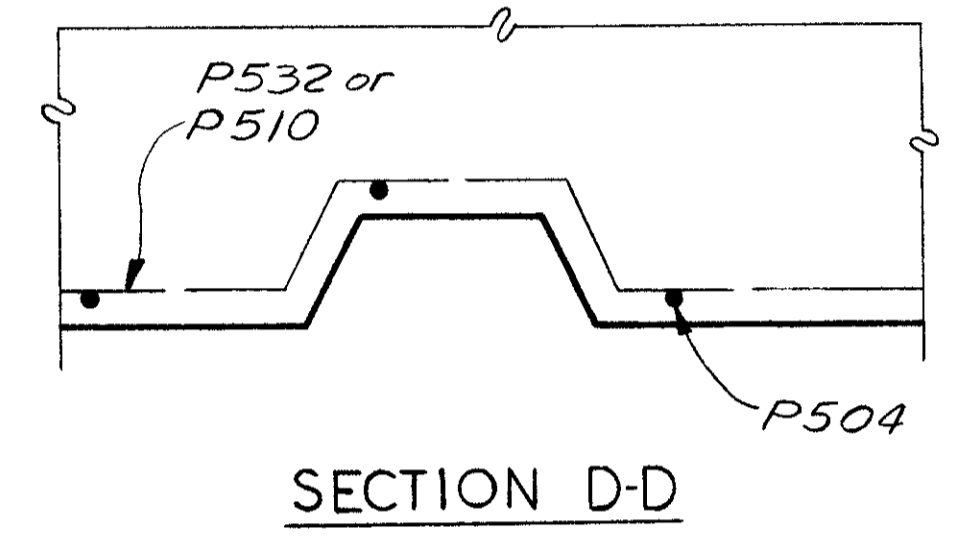
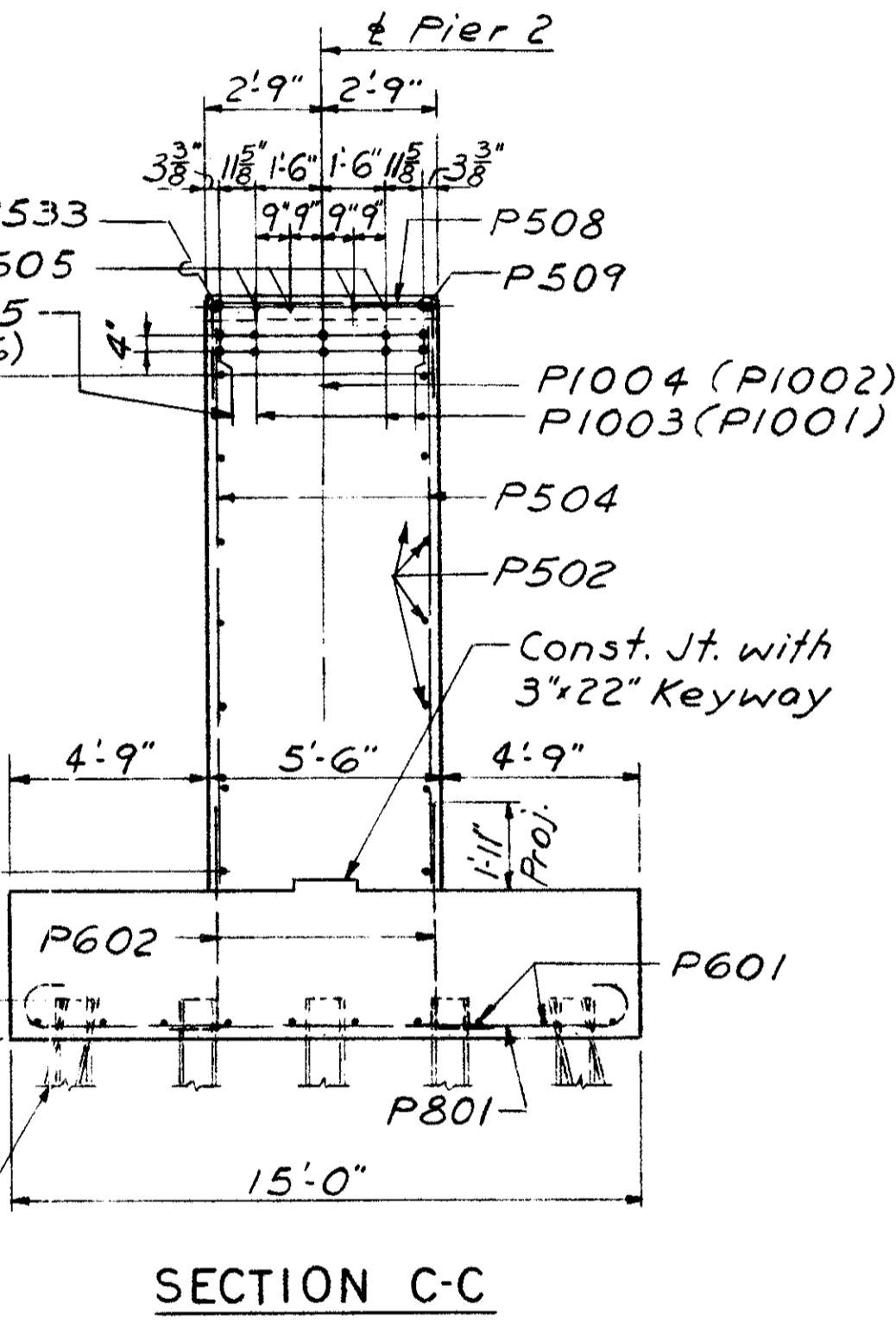
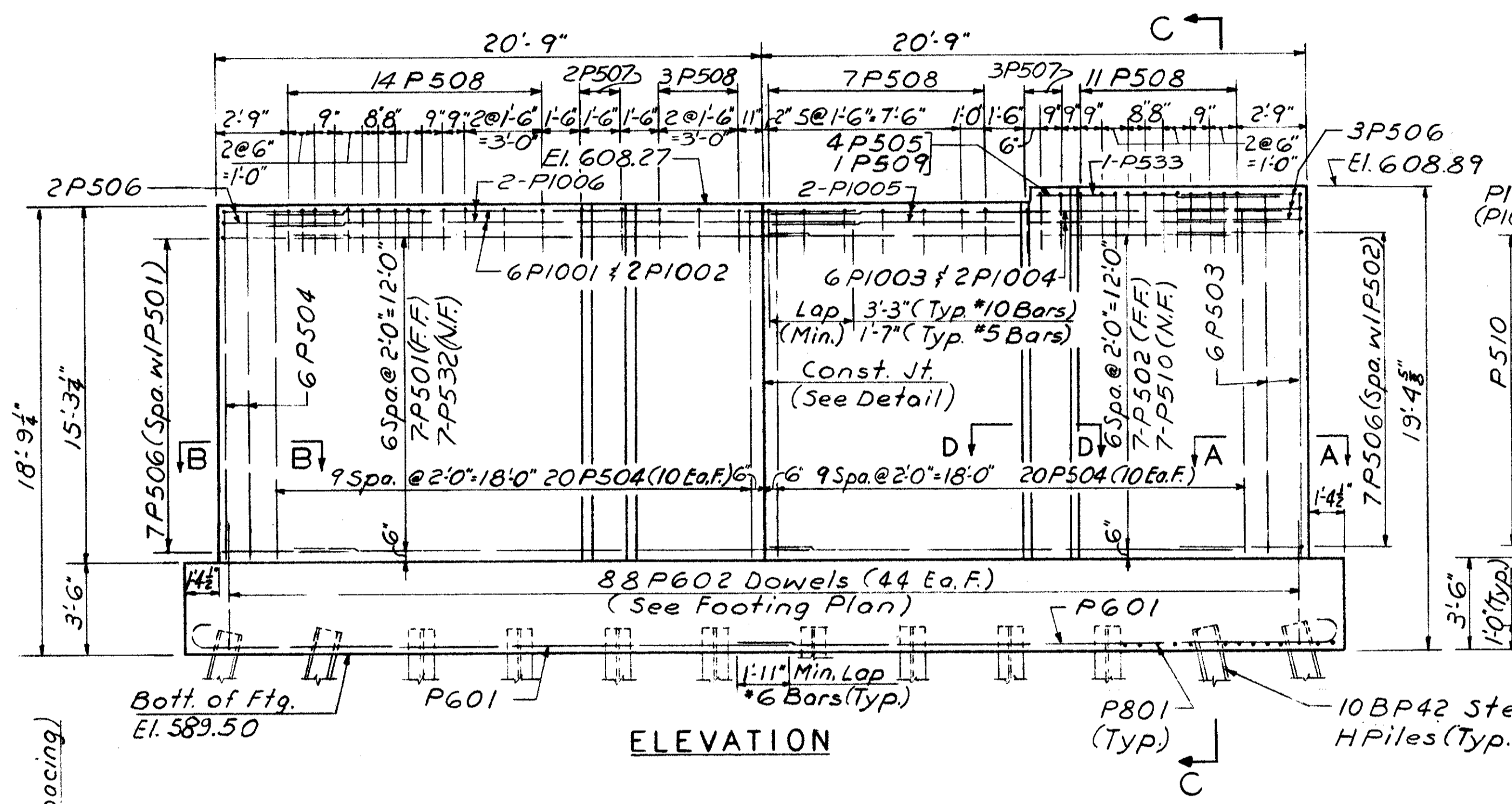
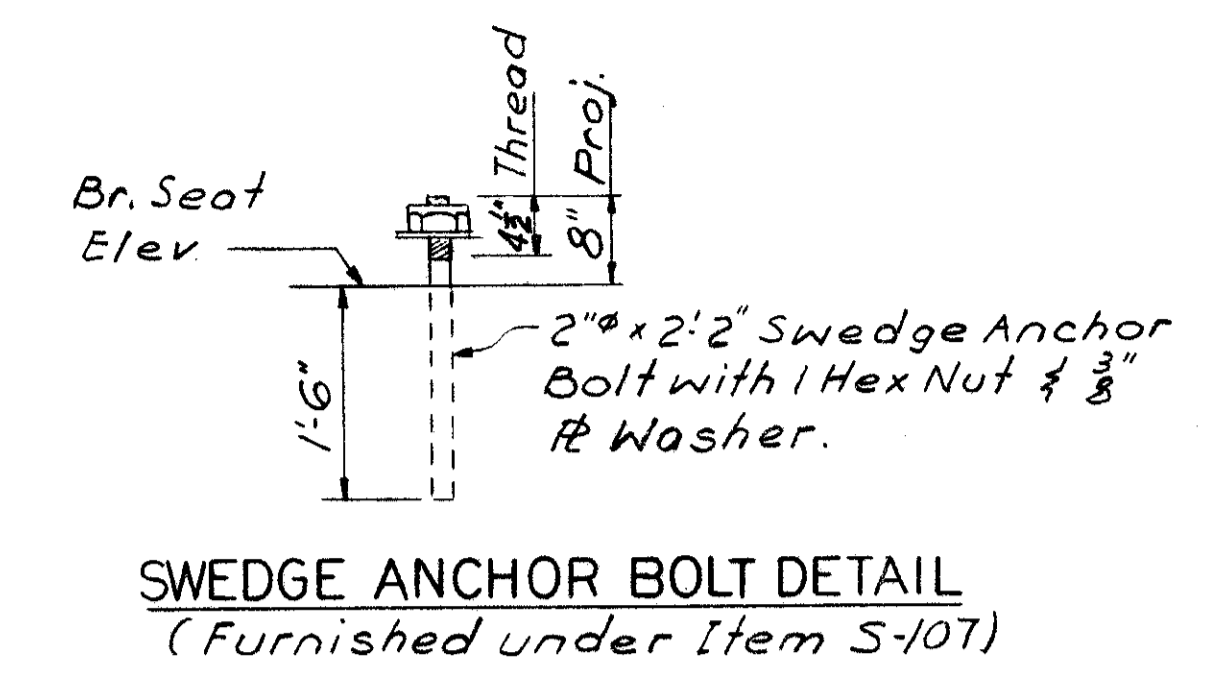
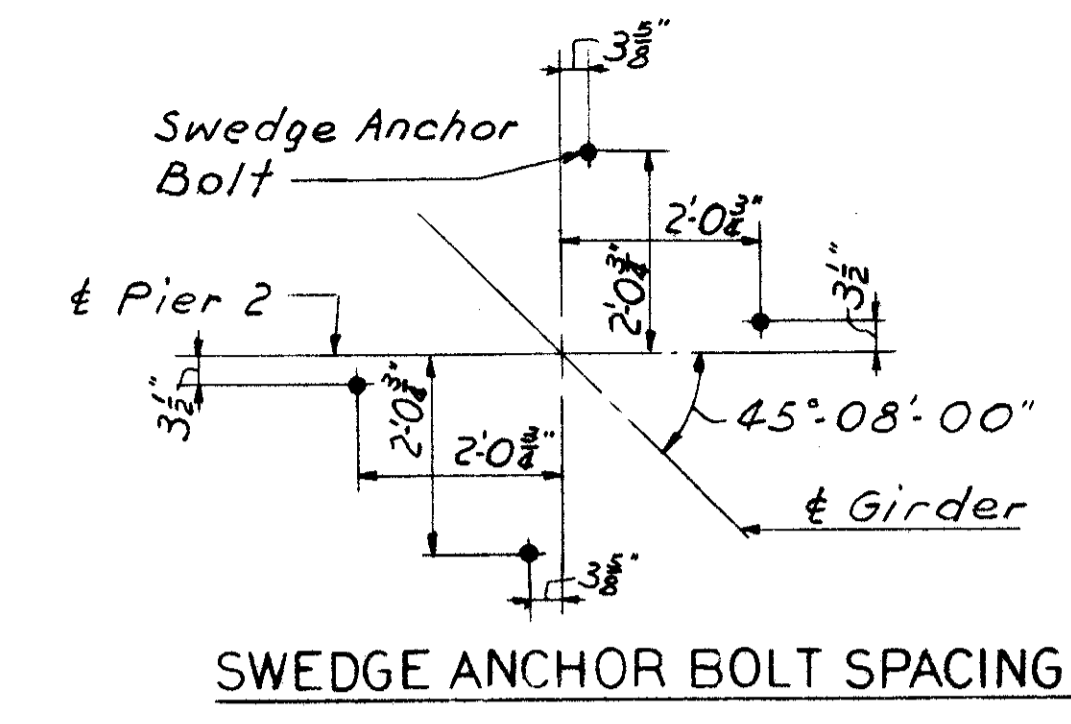
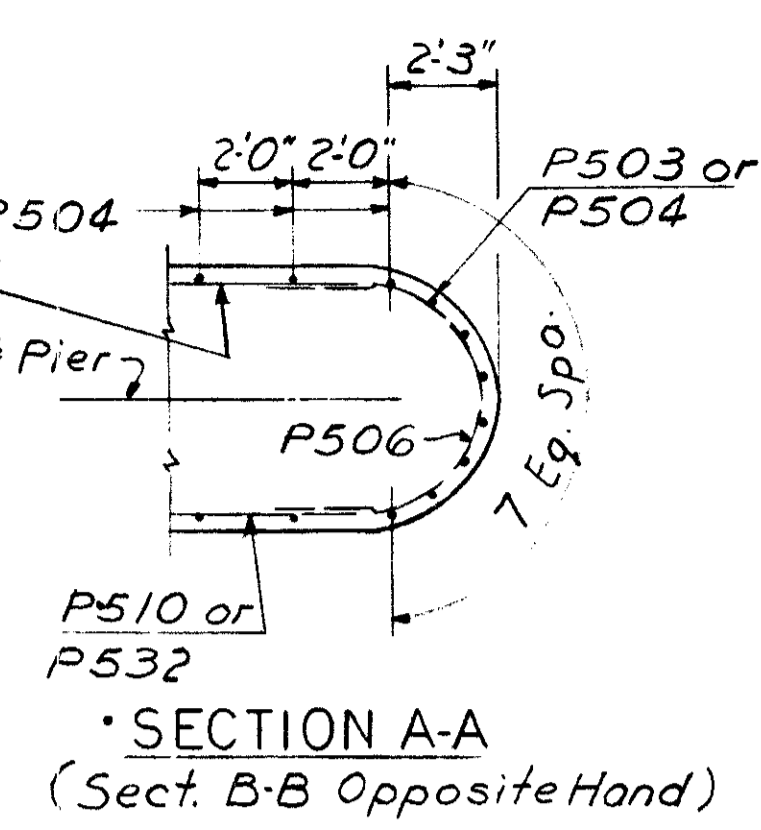
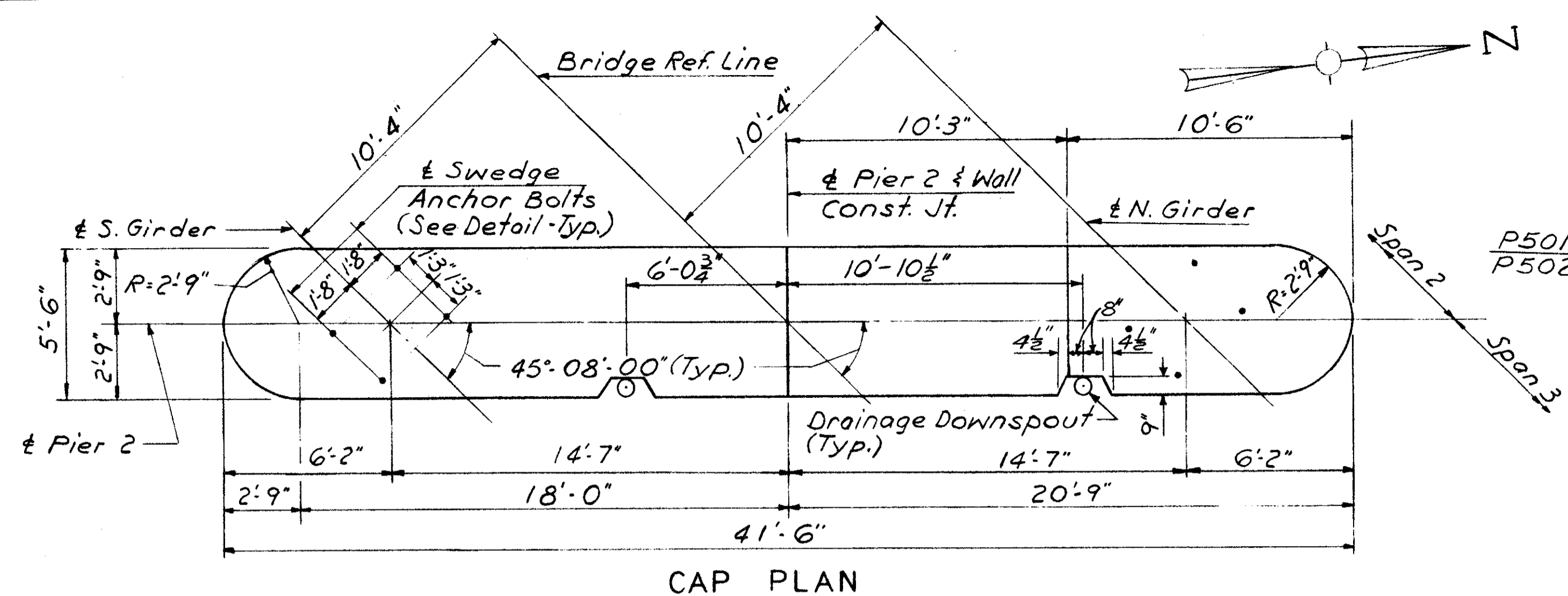
PIER I
BRIDGE NO. HAM-71-0207

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
S.U.	S.U.		D.W.	JHO 5/22/65	

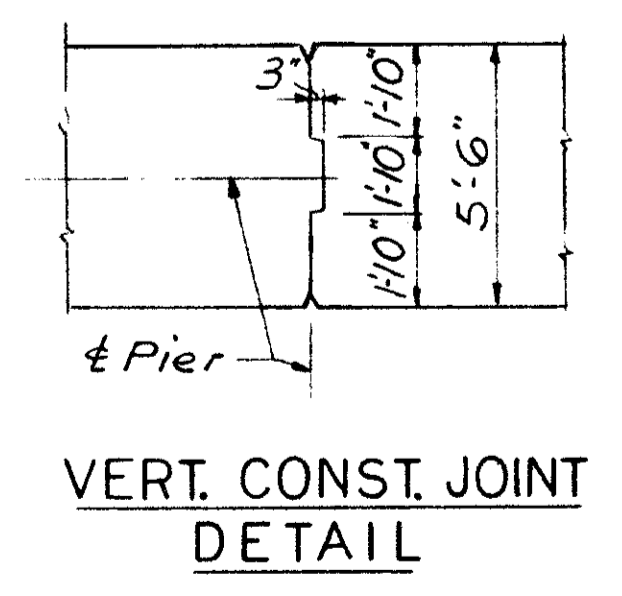
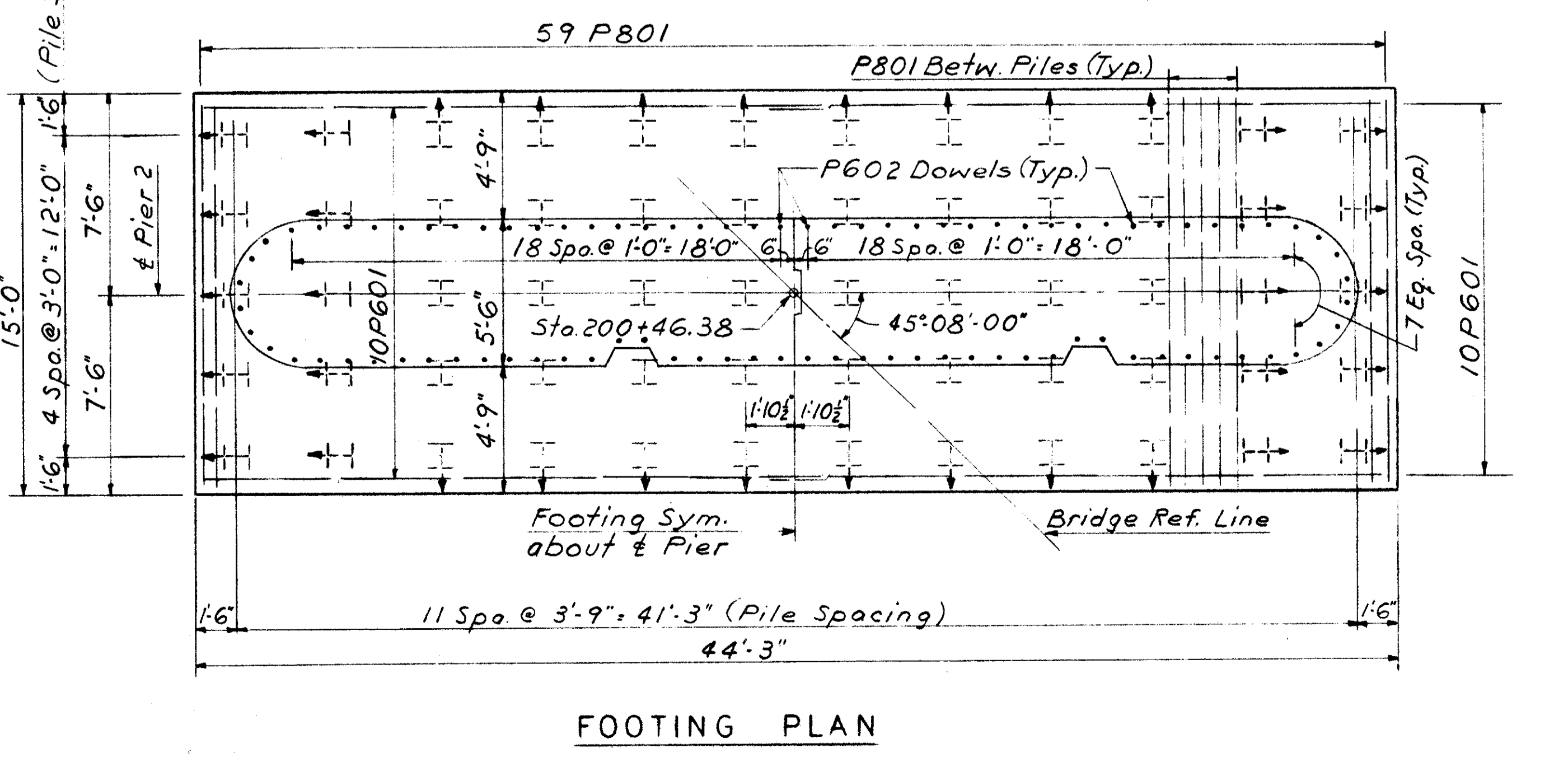
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

422
492

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

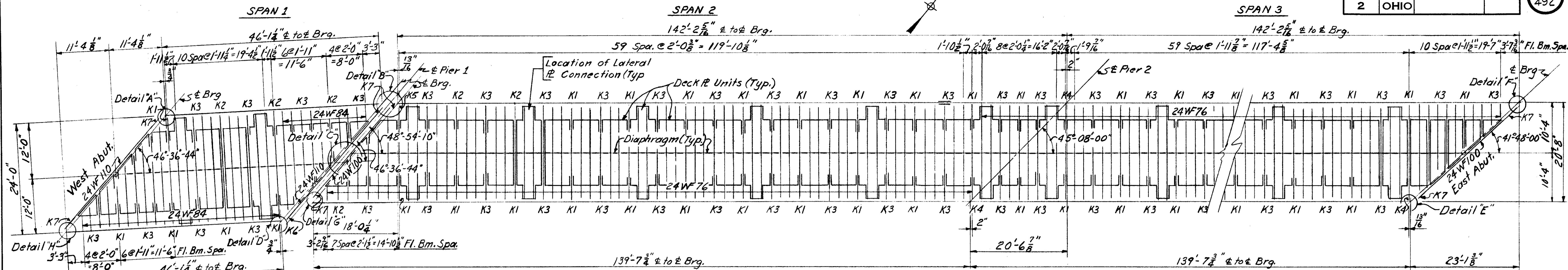


Notes:
Provide 3" clearance to reinforcing steel in footing and 2" in wall (Min). Concrete in footing shall be class 'E' & Concrete in wall shall be class 'C'. Special care shall be taken in placing reinforcing steel in the vicinity of the bridge seat so as to avoid interference with the drilling of swedge anchor bolt holes. All Piles shall be 10BP42 Steel HPiles. H denotes Vertical Piles. H± denotes Batter (4:12) Piles Ea.F. denotes Each Face.



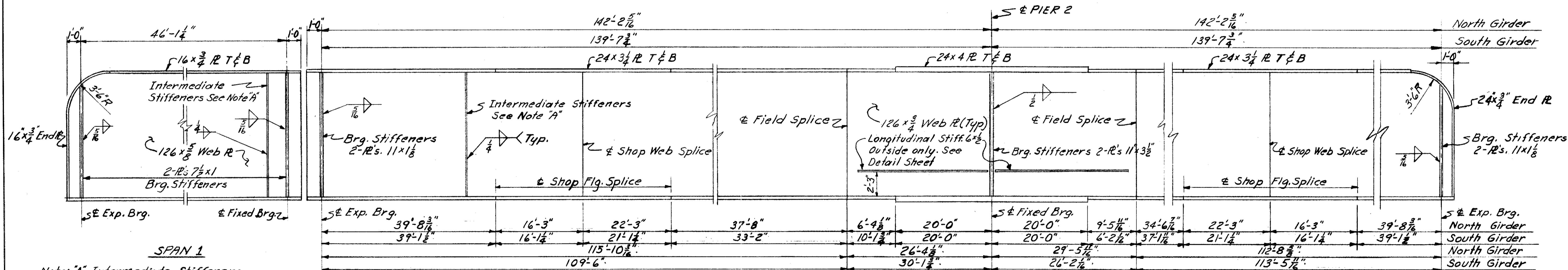
MICROFILMED
FEB 20 1985

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
PIER 2					
BRIDGE NO. HAM-71-0207					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
S. V.	S. V.		D. W.	JH 1-5-65	5/22/65



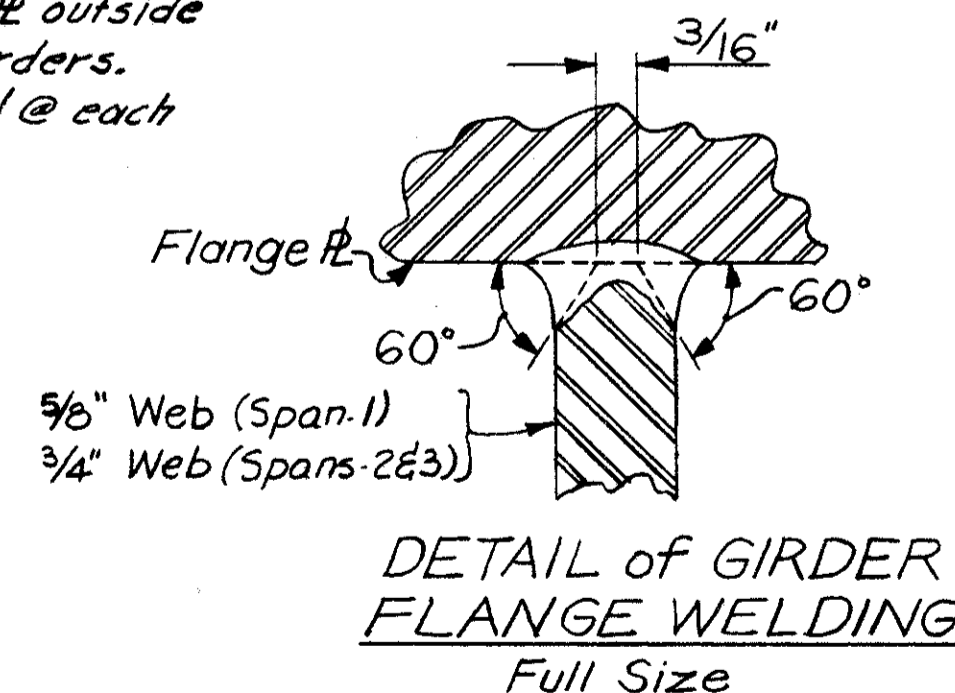
PLAN

Note: For Details of Deck Plates including lateral R connections. See Deck Details.



ELEVATION

Note: "A" Intermediate Stiffeners in spans 1, 2 & 3 to be 7x 1/2 R outside & 7 1/2 x 1/2 R on inside of girders. Stiffeners shall be spaced @ each Bracket K1, K2, K3 & K4



LOADING DATA

DEAD LOAD (#/Lin. Ft Track)	SPAN 1	SPANS 2 & 3
Track & Fastening	200	200
Ballast	2820	2300
Asph. P.C. Concrete Deck Protection	540	440
Steel Weight	2990	3310
Total Dead Load	6550	6250

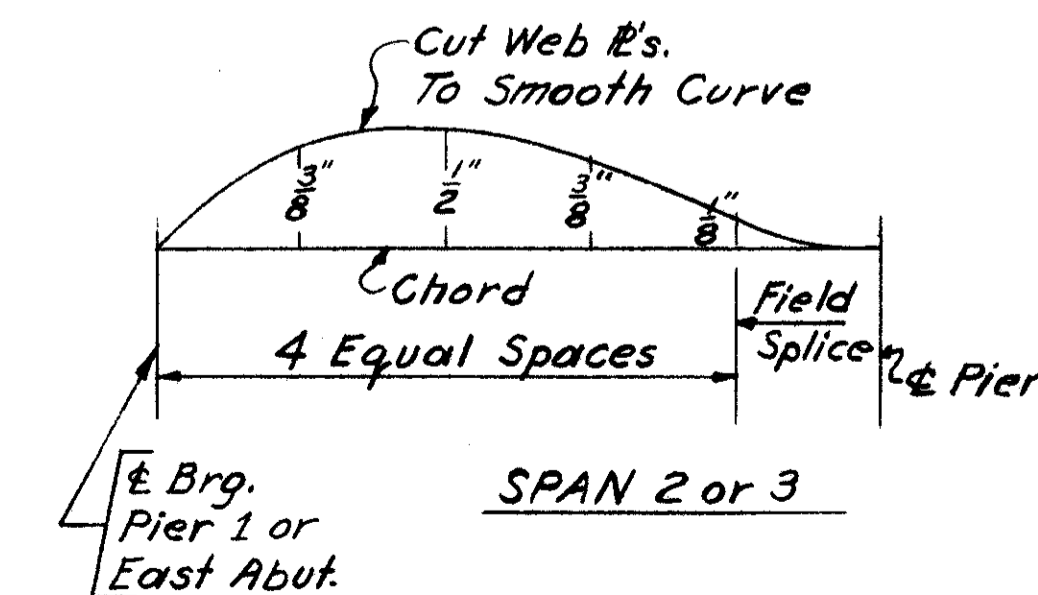
LIVE LOAD

Coopers E72 & Diesel Impact (Without Hammer Blow)

ITEM	UNIT	DESIGN DATA			INTERIOR FLOOR BEAMS	
		SPAN 1	SPANS 2 & 3		SPAN 1	SPANS 2 & 3
			POSITIVE	NEGATIVE		
Dead Load Moment (Ft.-Kip)		787	4020	8360	38	28
Live Load Moment (Ft.-Kip)		1550	8750	10800	157	127
Impact Moment (Ft.-Kip)		619	2290	2820	67	56
Misc. (Eccen. & Centrif. Force) Mom. (Ft.-Kip)		42	-	-	2	8
Total		2998	15060	21980	264	219
Dead Load Shear (Kip)		72		250	5.6	4.5
Live Load Shear (Kip)		165		400	17.5	16.2
Impact Shear (Kip)		67		105	7.5	7.2
Misc. Shear (Kip)		4		-	0.5	1.0
Total Shear		308		755	31.1	28.9
Net I of Section (in ⁴)		200,200	775,000	935,000	2111	1870
Flange Stress (Ksi)		11.5	15.4*	18.9	18.1	16.9
Max Shear Stress (Ksi)		3.9	-	8.0	2.8	2.8

*Section Governed by A.R.E.A. Allow. Live Load Deflection

Notes:
For Joint Preparation for Submerged Arc Weldments see "Typical Structural Steel Details" Sheet No. 444
Intermediate and bearing stiffeners, floorbeam connection plates, girder end plates and all field attached sections shall be set at right angles to the girder flanges.
The shop welded web and flange splices may be moved a maximum of 3'-0" to suit the material to be used by the fabricator.



CAMBER FOR GIRDERS
No Camber for Span 1 Girders

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CINCINNATI, OHIO

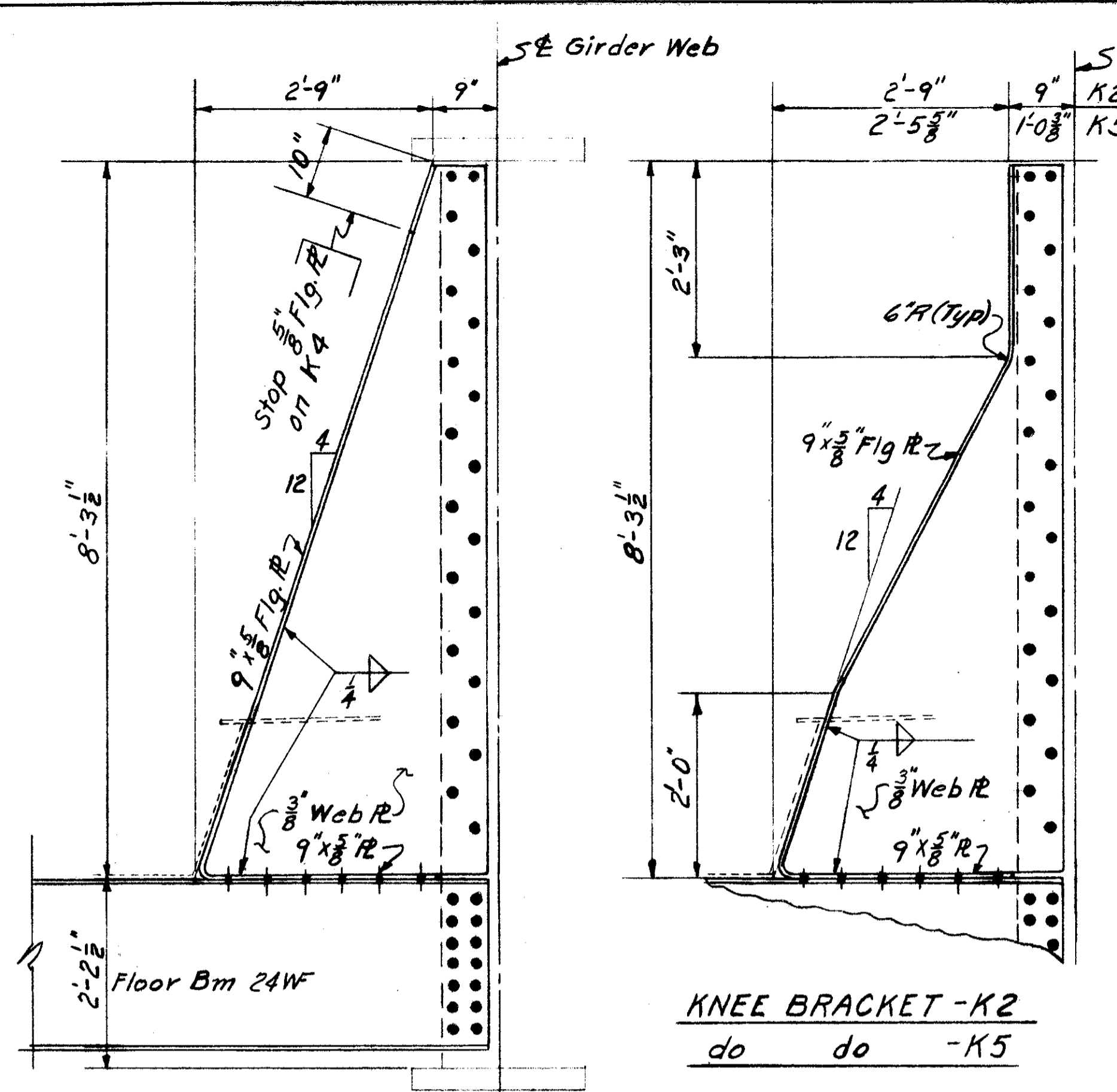
STRUCTURAL STEEL DETAILS

BRIDGE NO. HAM-71-0207

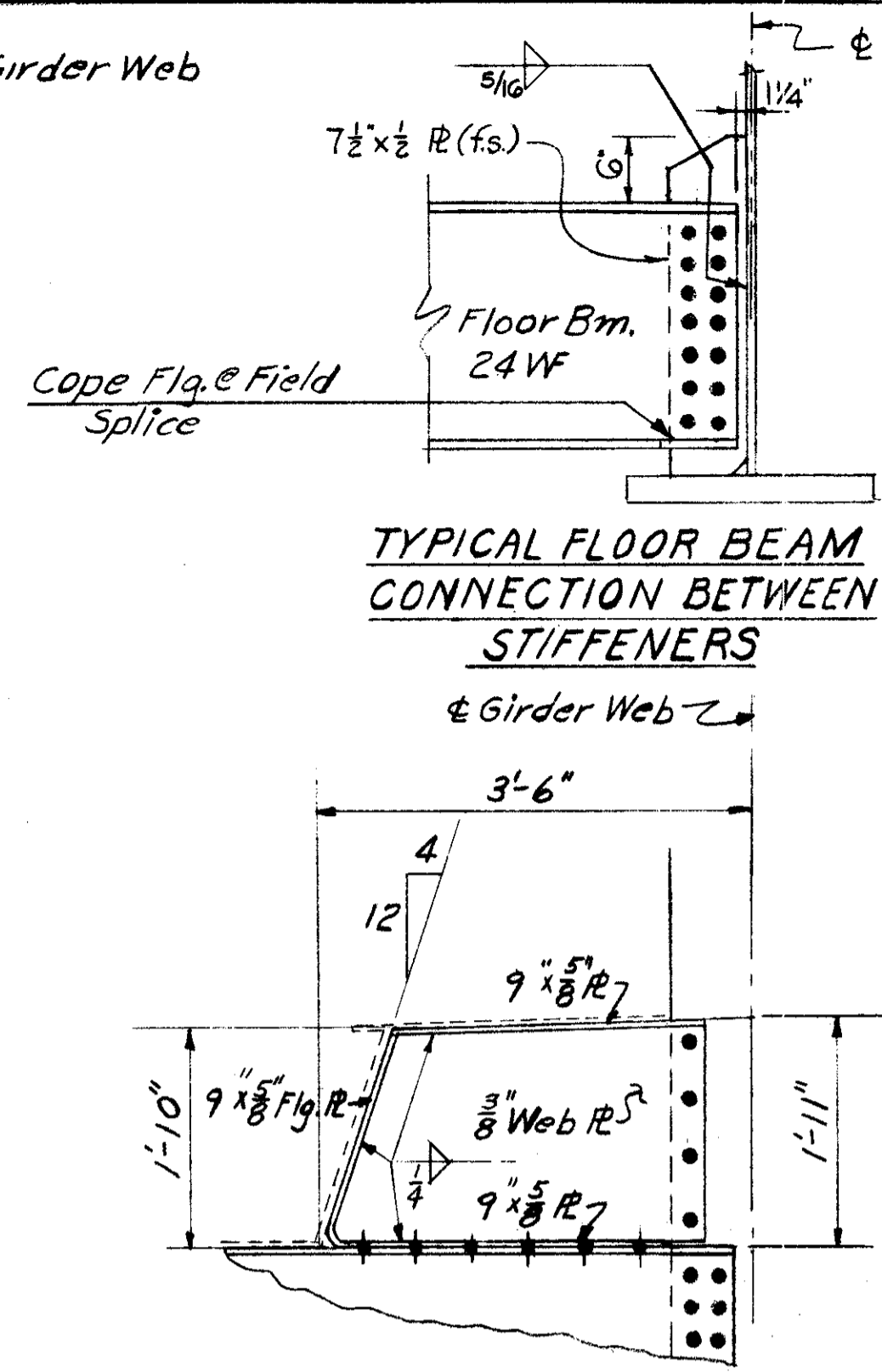
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
WFB	HWT		R.L.K. 1-7-65	JHO 5/22/65	

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FEB 20 1985

**HAMILTON COUNTY
HAM-71-(1.56) (2.51)**

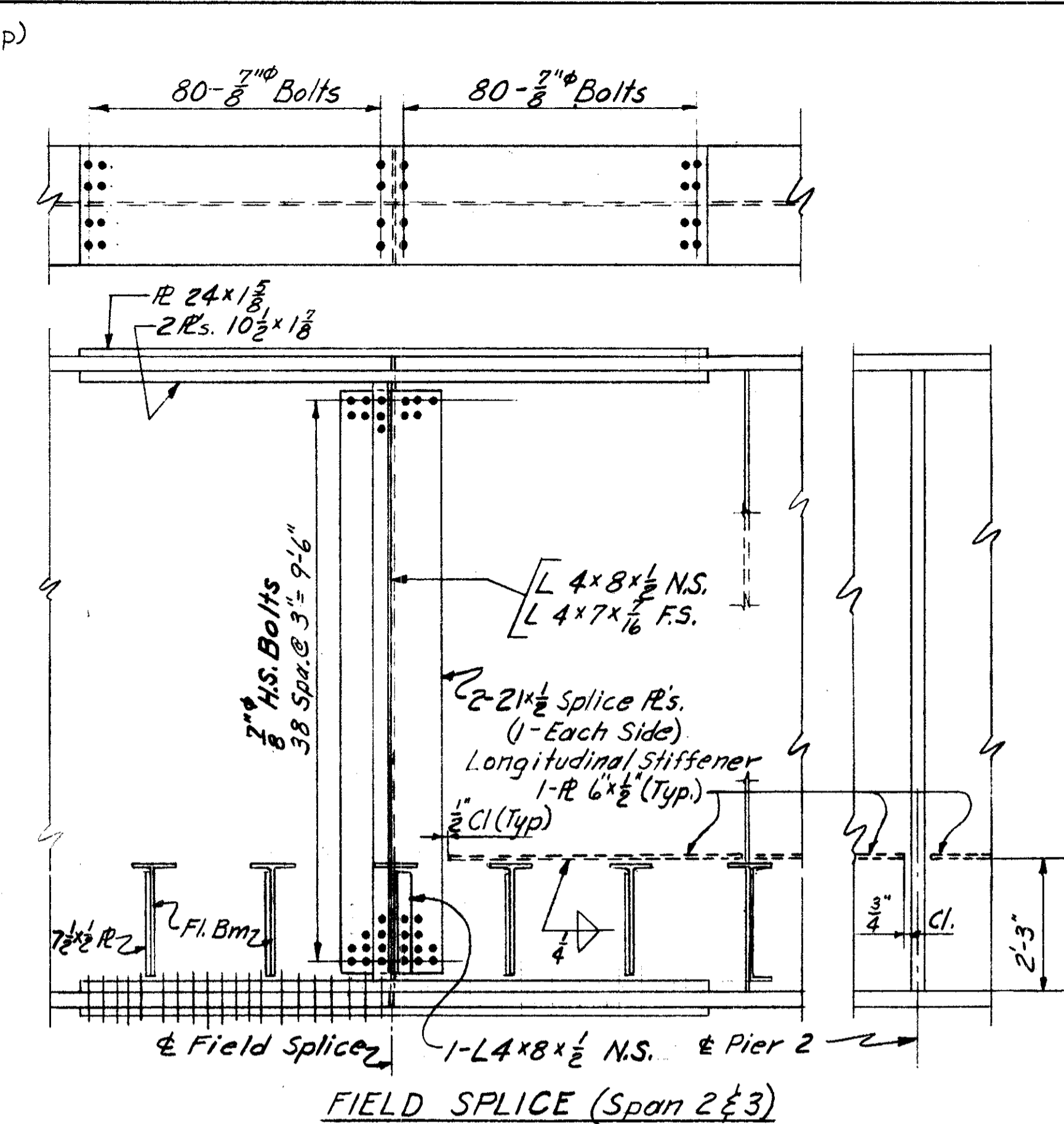


KNEE BRACKET-K2
do do -K5

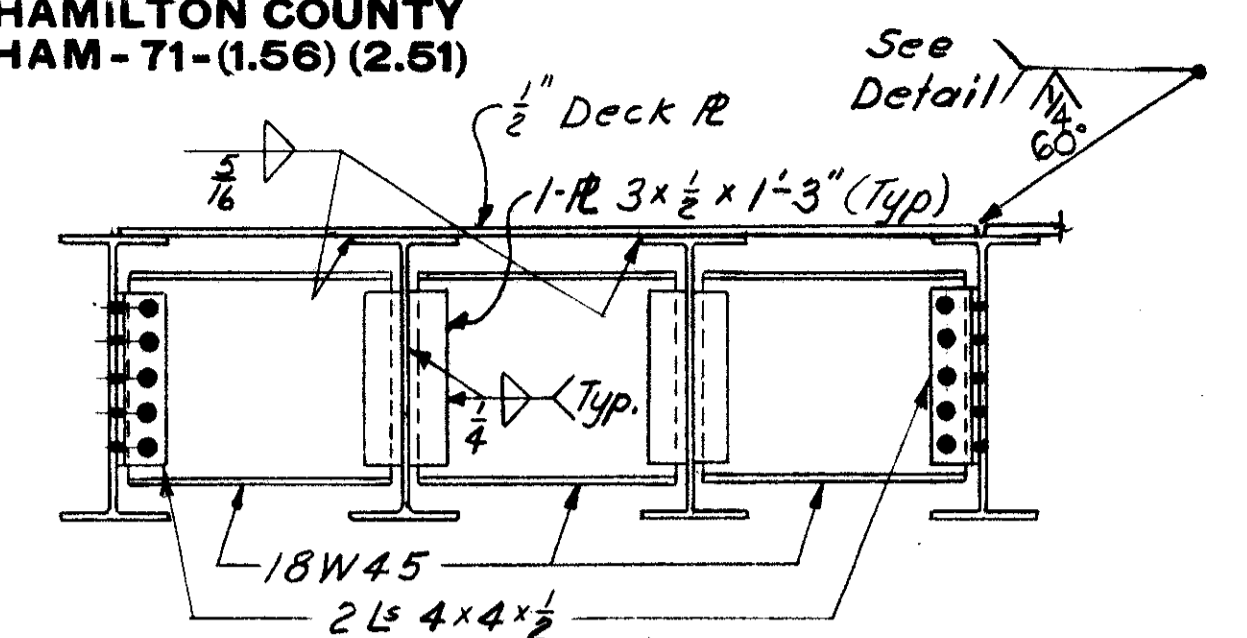


**TYPICAL FLOOR BEAM
CONNECTION BETWEEN
STIFFENERS**

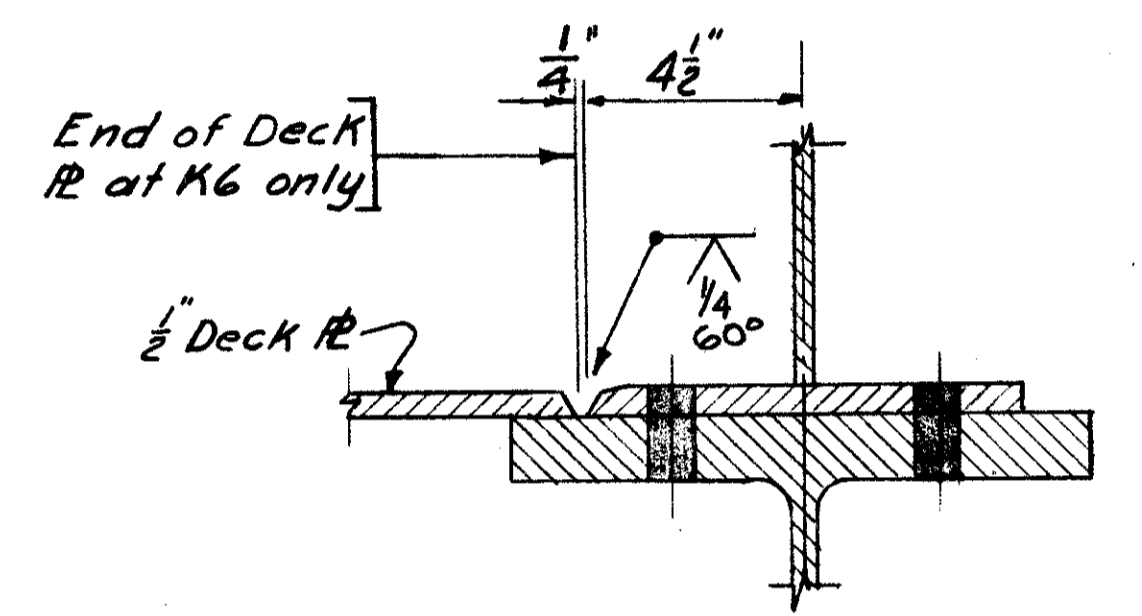
BRACKET-K3



FIELD SPLICE (Span 2 & 3)



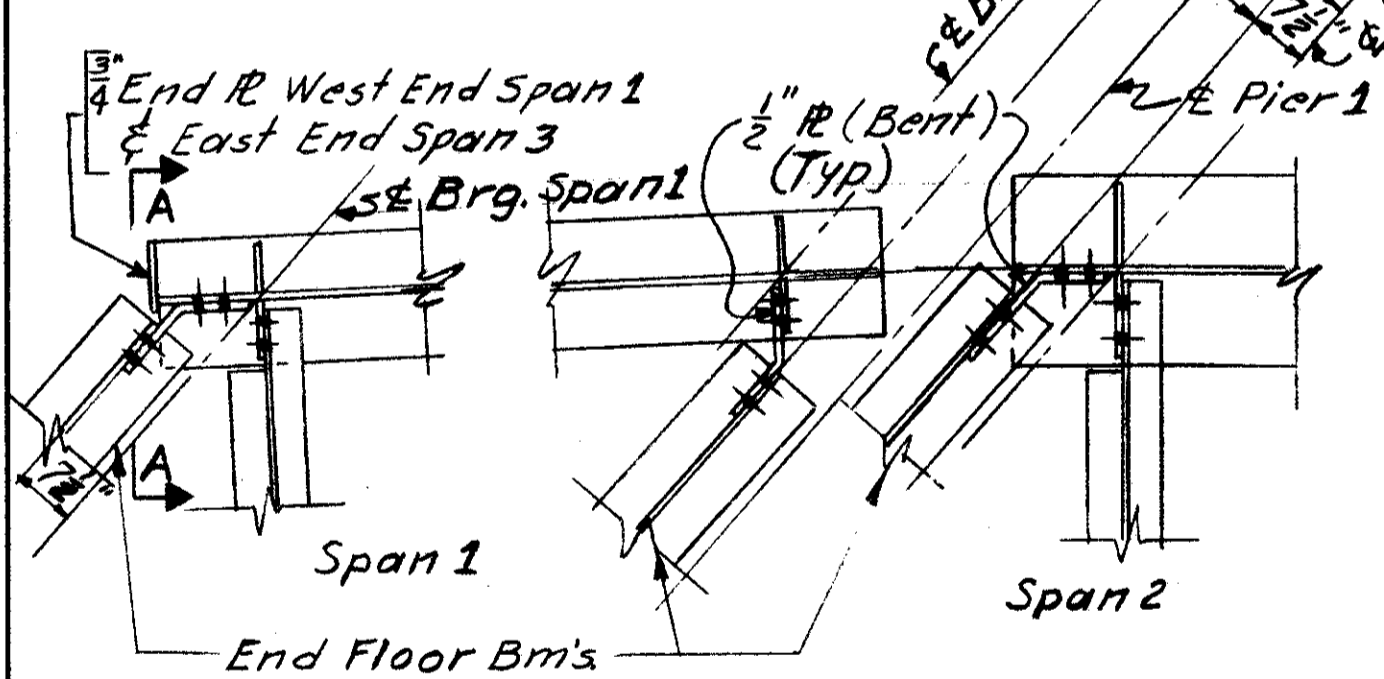
DIAPHRAGMS & FLOOR BEAM DETAILS
Normal unit shown. Special units can have different numbers of floor beams at option of fabricator.



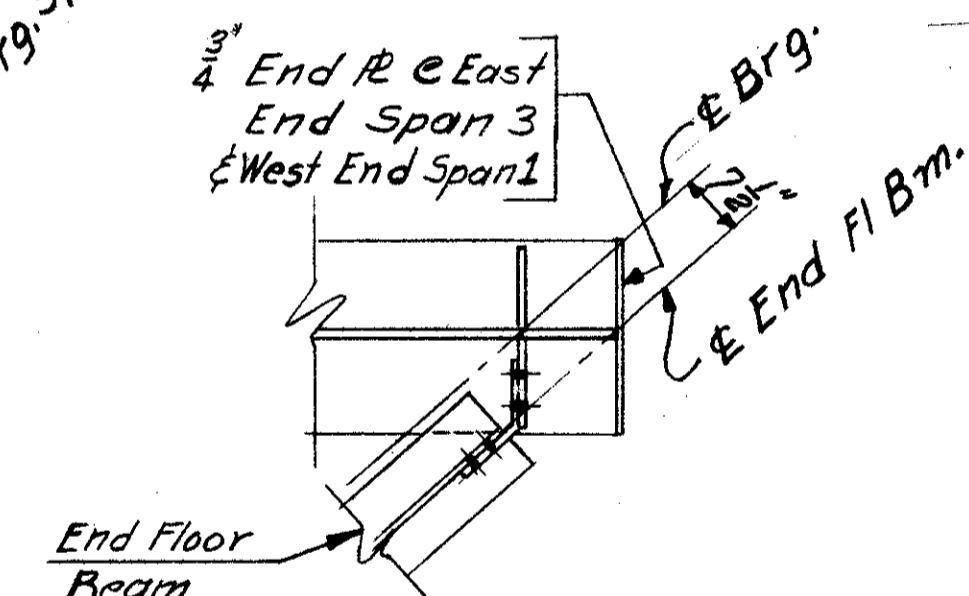
DETAIL 'J'

Note: Typical Floor Beam Connection at Stiffeners.

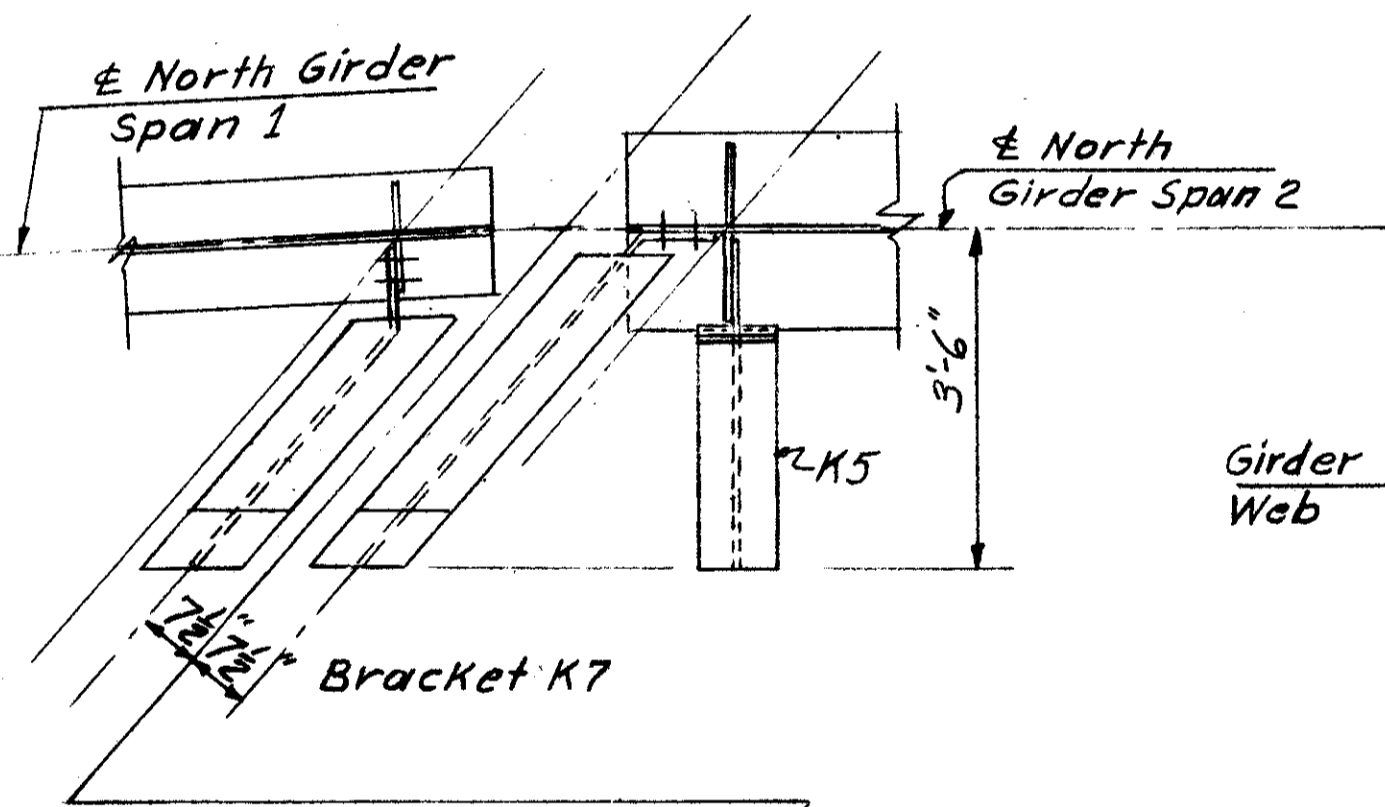
KNEE BRACKET-K1
do do -K4



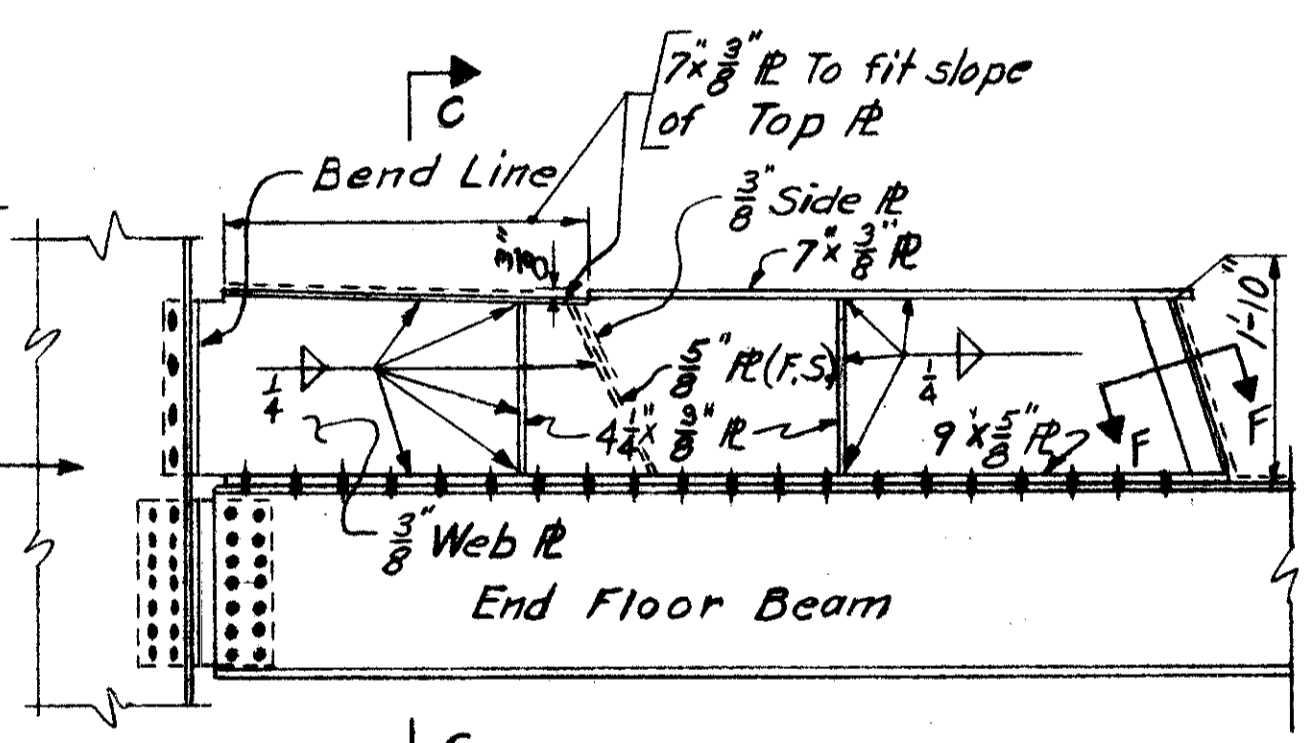
DETAIL 'A'
DETAIL 'B'
DETAIL 'D' Similar by Rotation
DETAIL 'E' Similar by Rotation



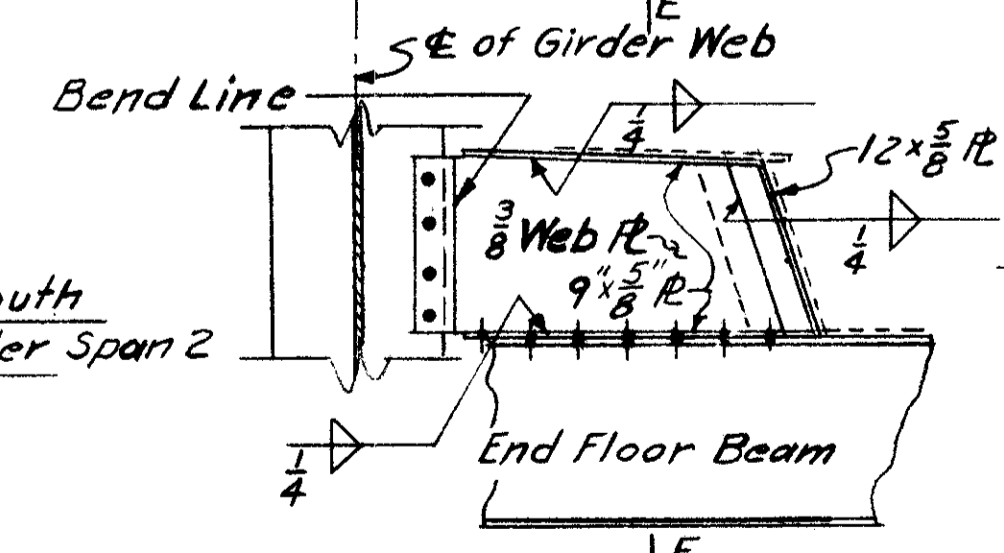
DETAIL 'F'
DETAIL 'G' & 'H'
(Similar by Rotation)



PART PLAN @ PIER 1
(Top Flange not Shown)

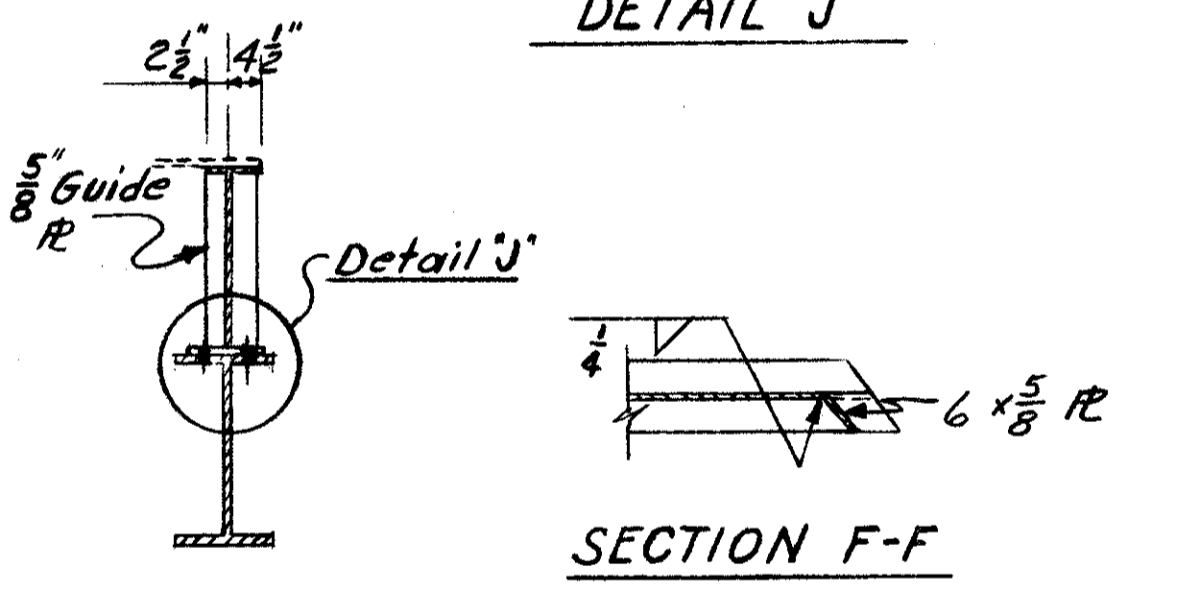


**SECTION B-B
BRACKET K6**

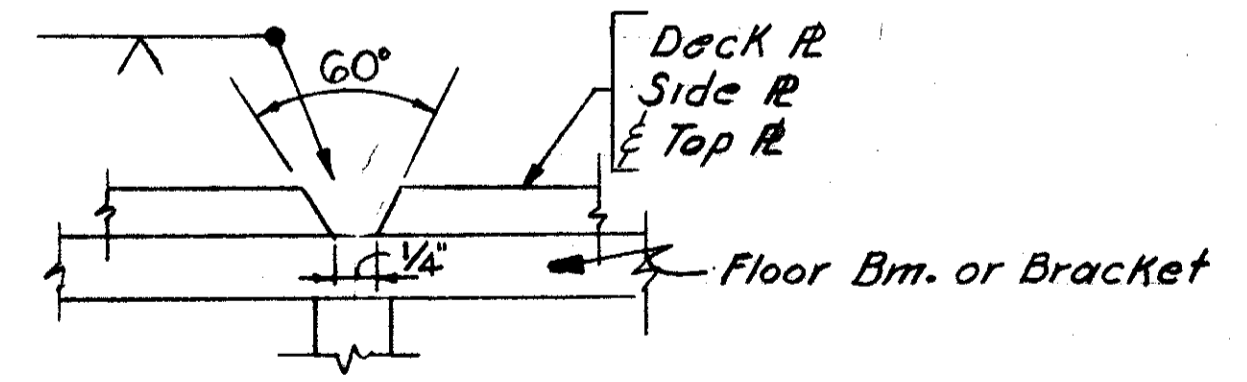


**SECTION D-D
BRACKET K7**

SECTION E-E



SECTION C-C



TYPICAL WELDING DETAIL
For Deck R's, Side R's & Top R's.

Note: Structural steel inaccessible for painting after erection shall be given final field coats prior to erection.

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CONSULTING ENGINEERS
CINCINNATI, OHIO

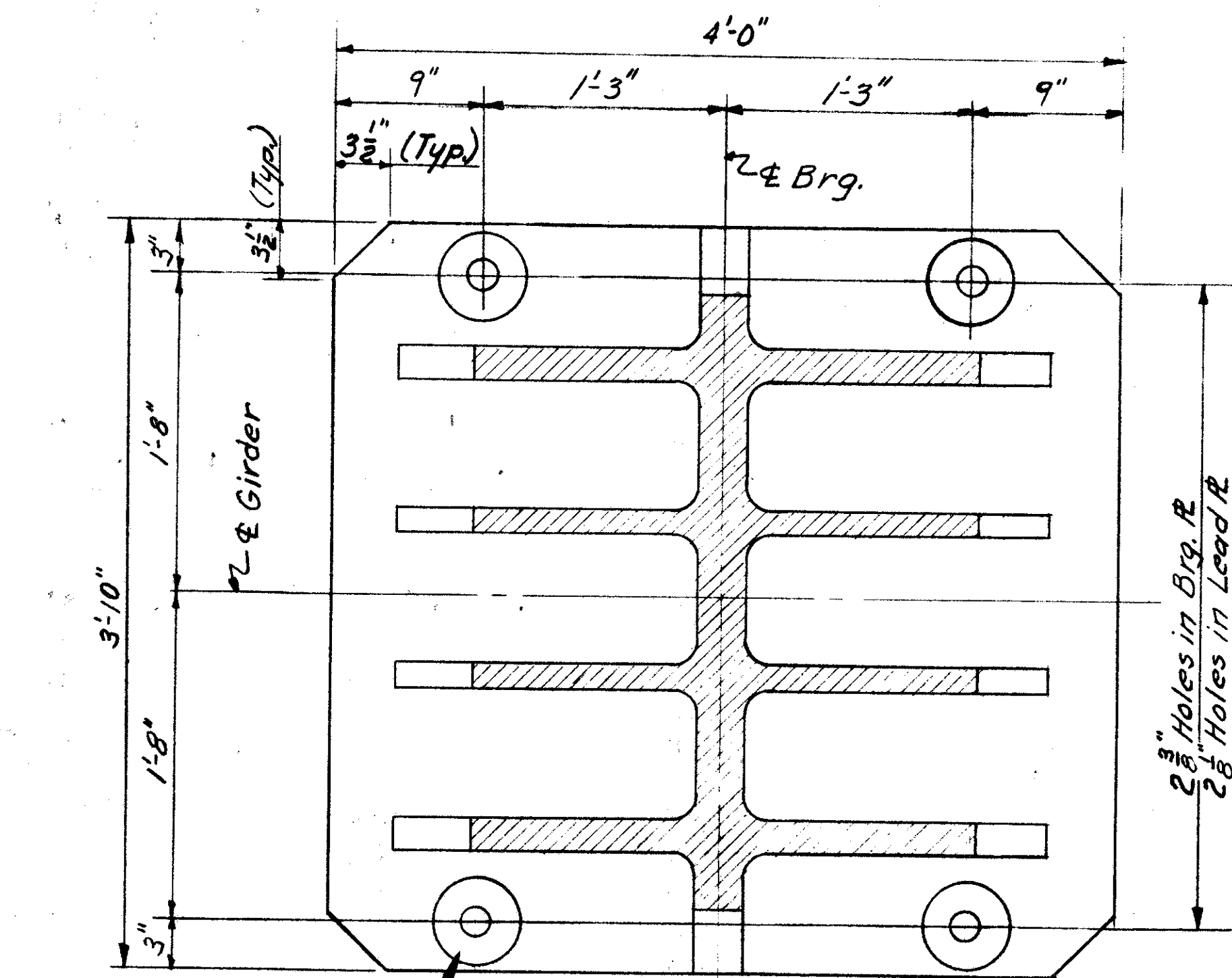
STRUCTURAL STEEL DETAILS
BRIDGE NO. HAM-71-0207

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
WFB	HWT.		R.L.K. 1-11-65	JHO 5/22/65	

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

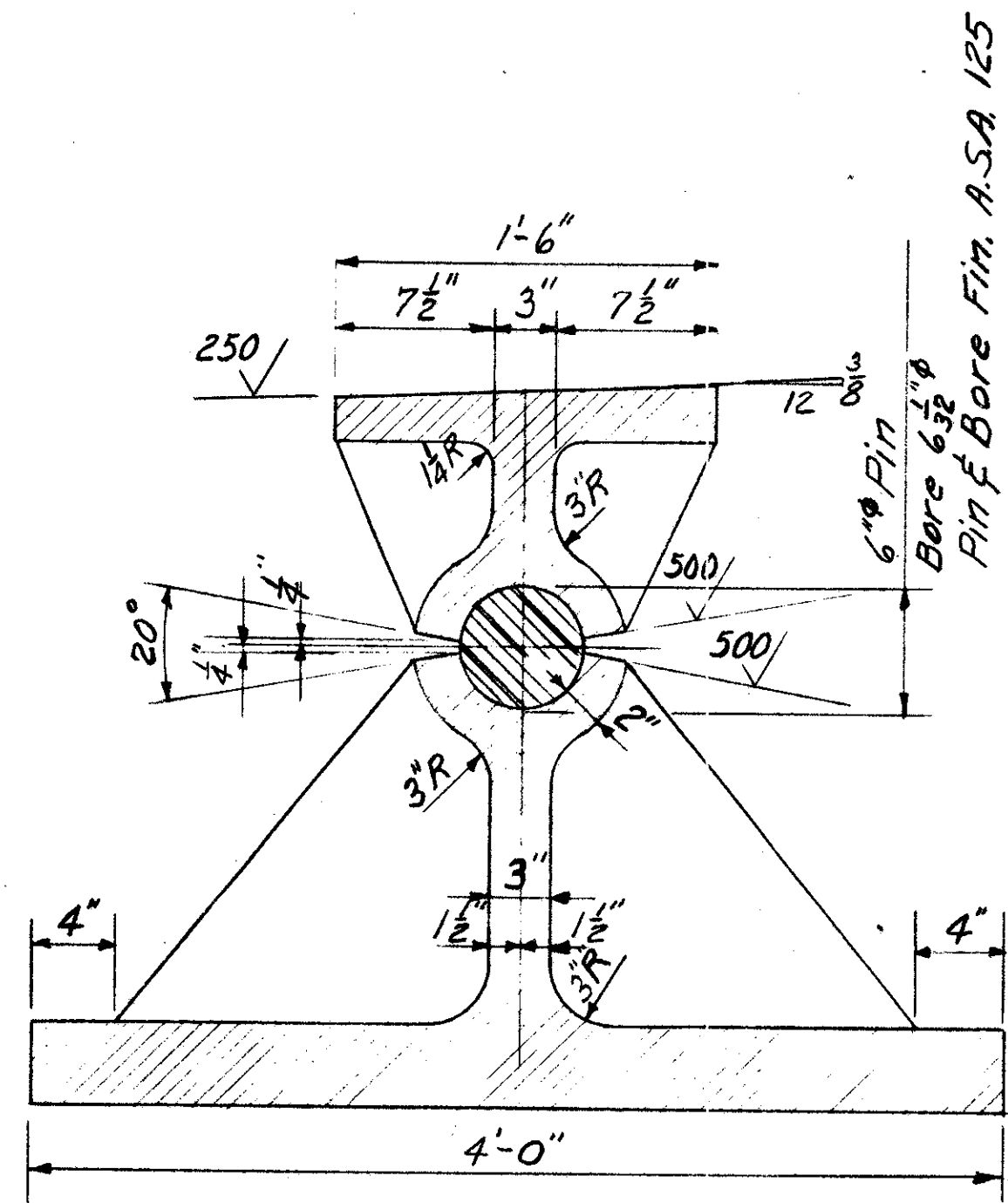
425
492

HAMILTON COUNTY
HAM-71-(1.66) (2.61)

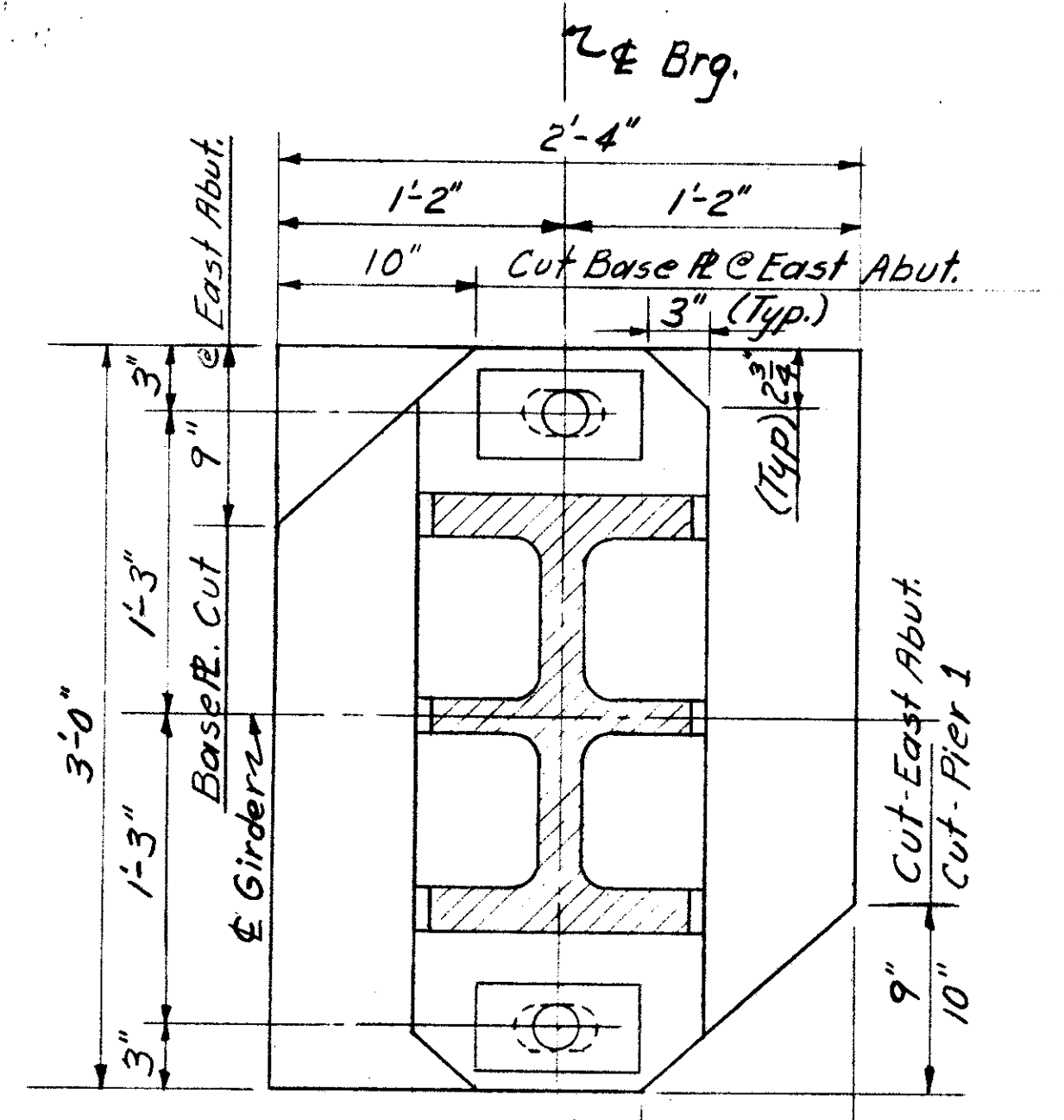


SECTION A-A

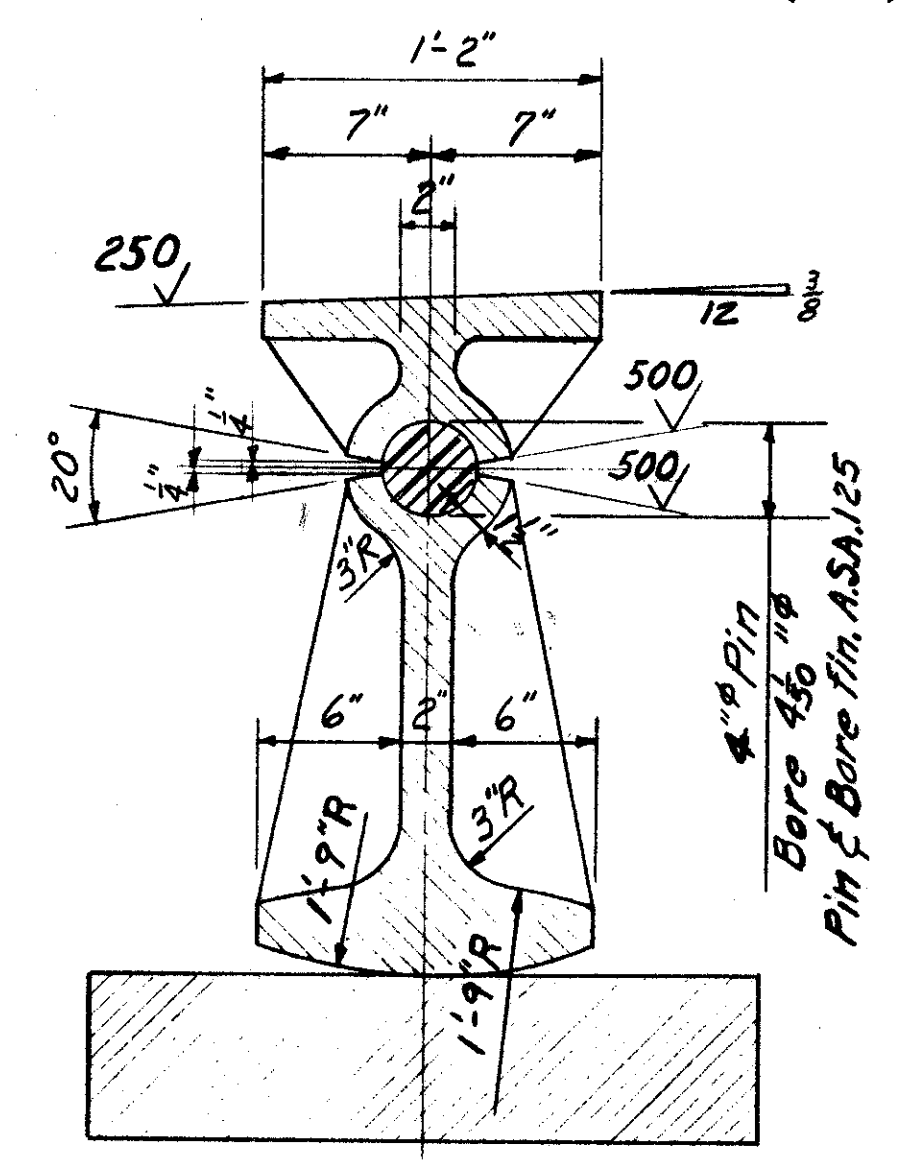
250/ 5/8" Spot Face (Typ)



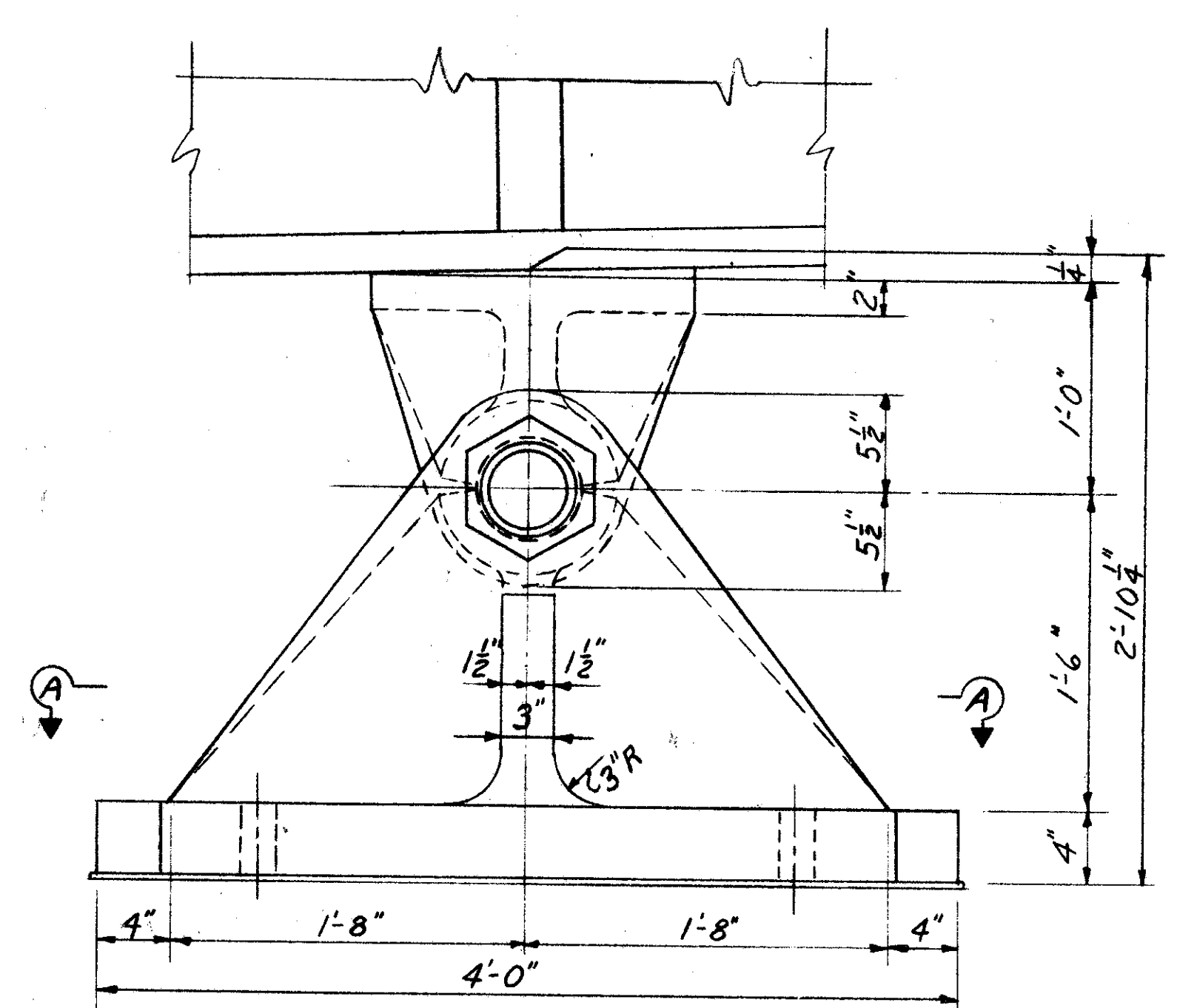
SECTION B-B



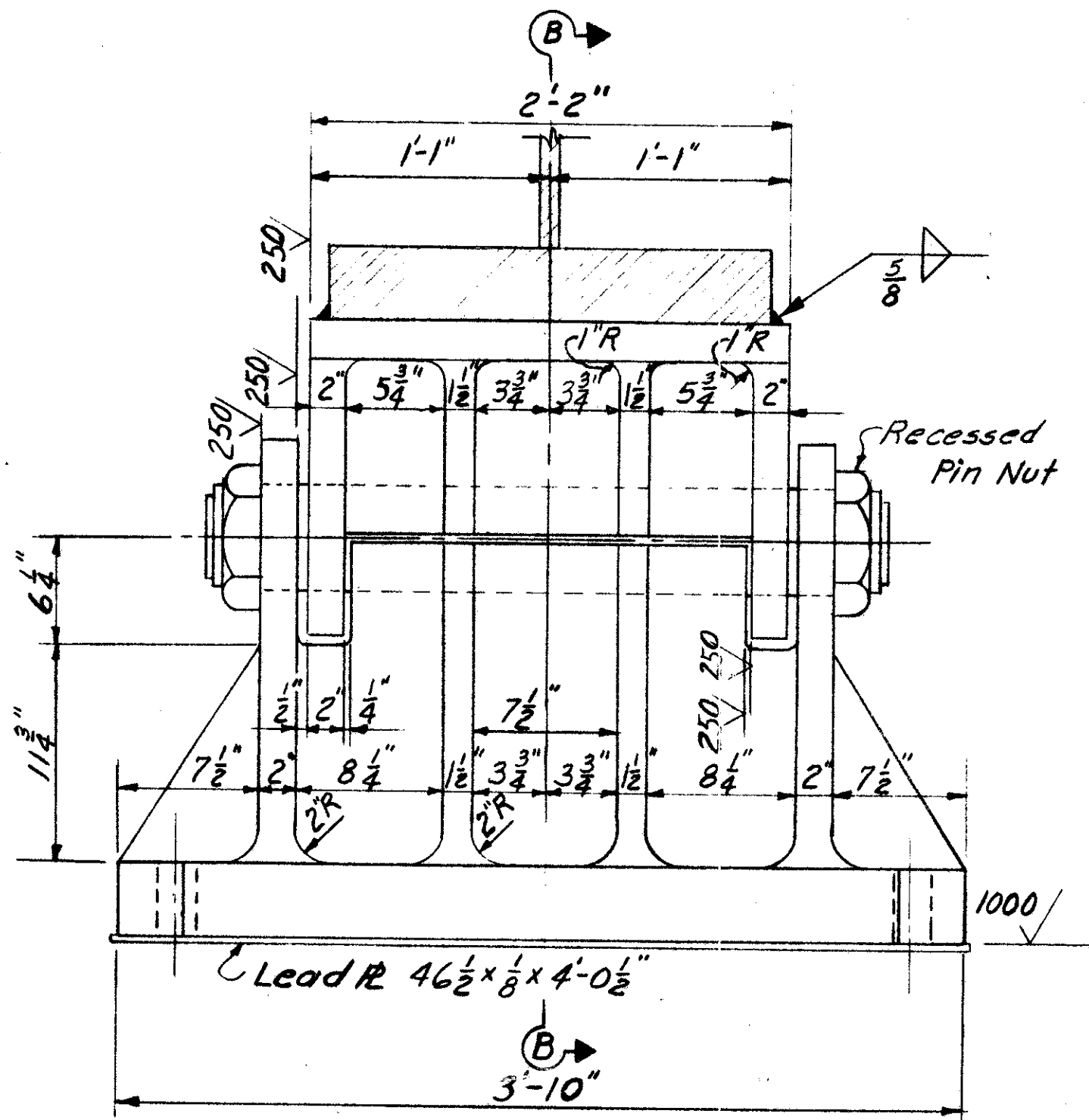
SECTION C-C



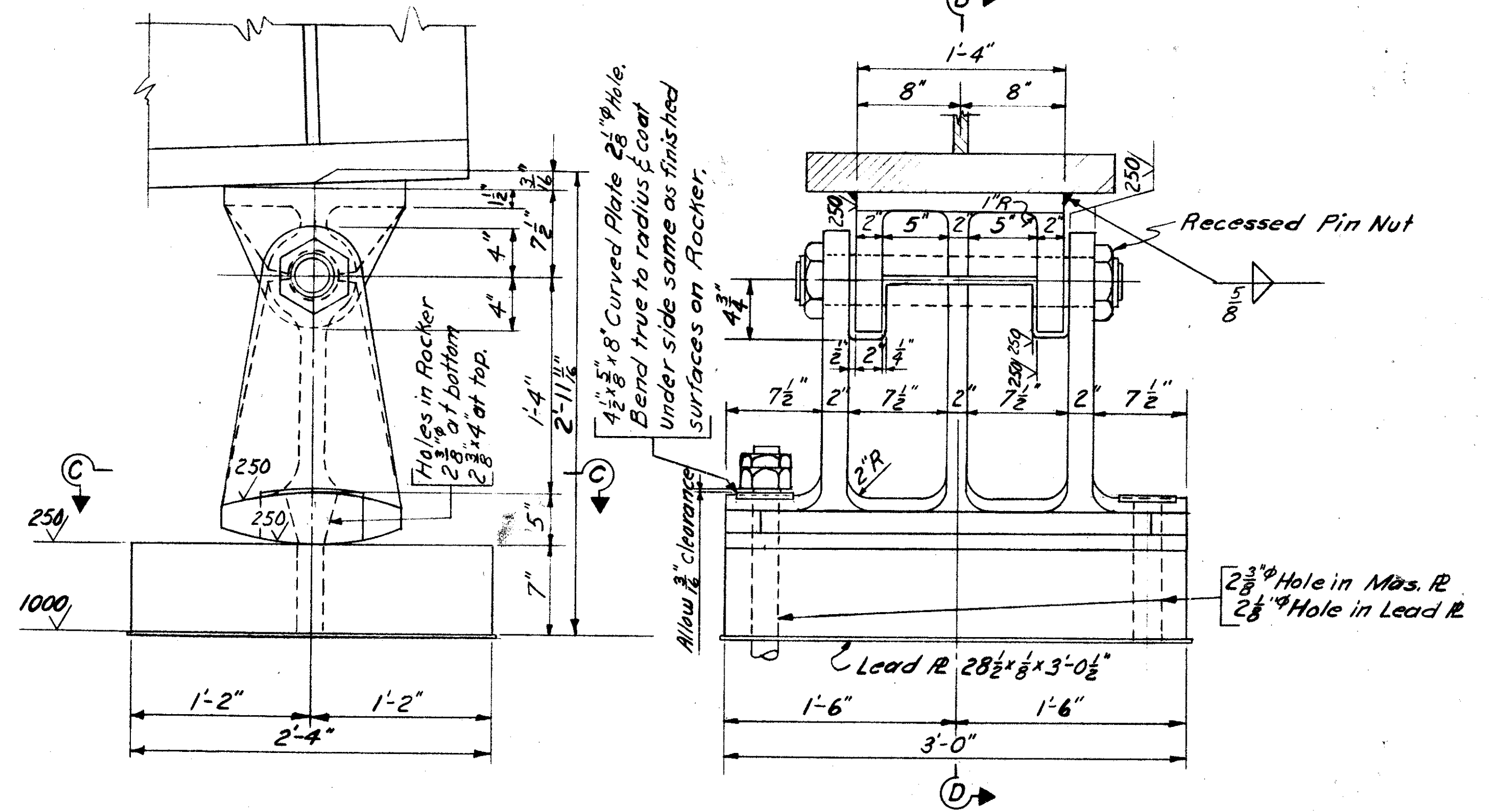
SECTION D-D



FIXED BEARING
(@ Pier 2)
(Cast Steel, 2 Req'd.)



EXPANSION BEARING
(@ Pier 1 & East Abut, Span 2 & 3)
(Cast Steel, 4 Req'd.)



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FEB 20 1965

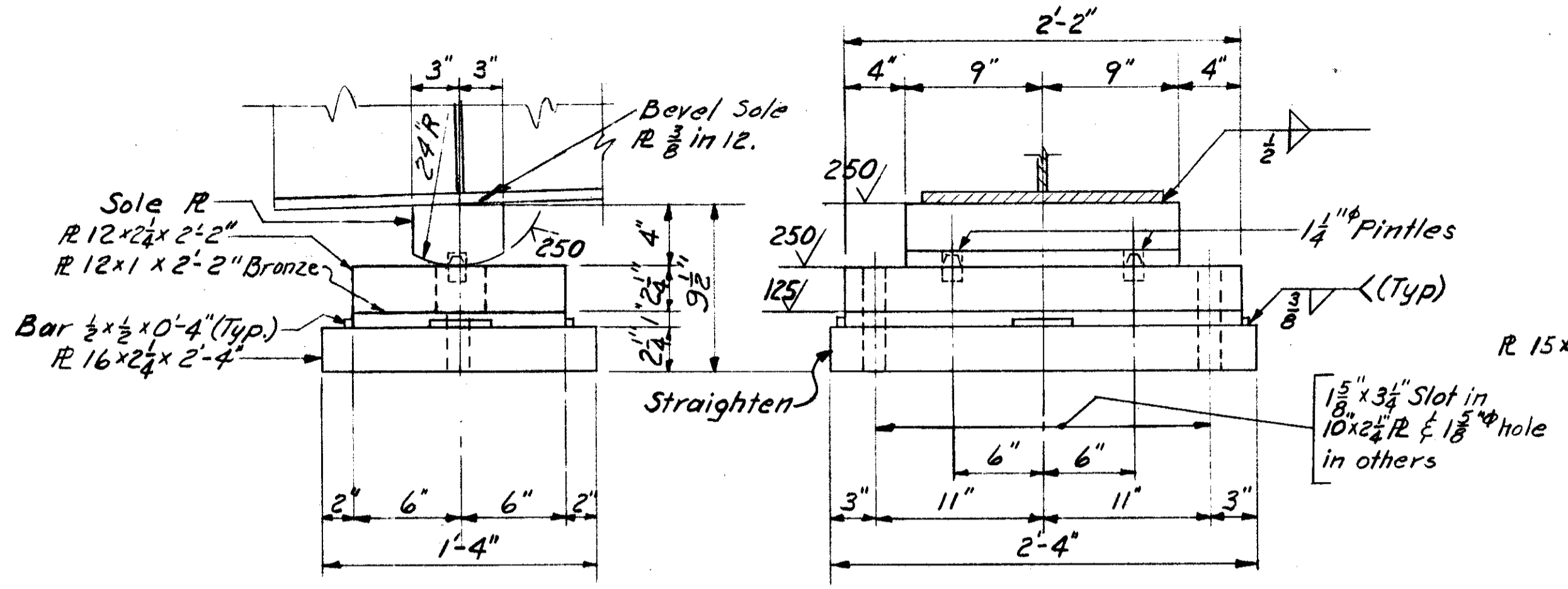
Work Sheets: 425 & 426 Together

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

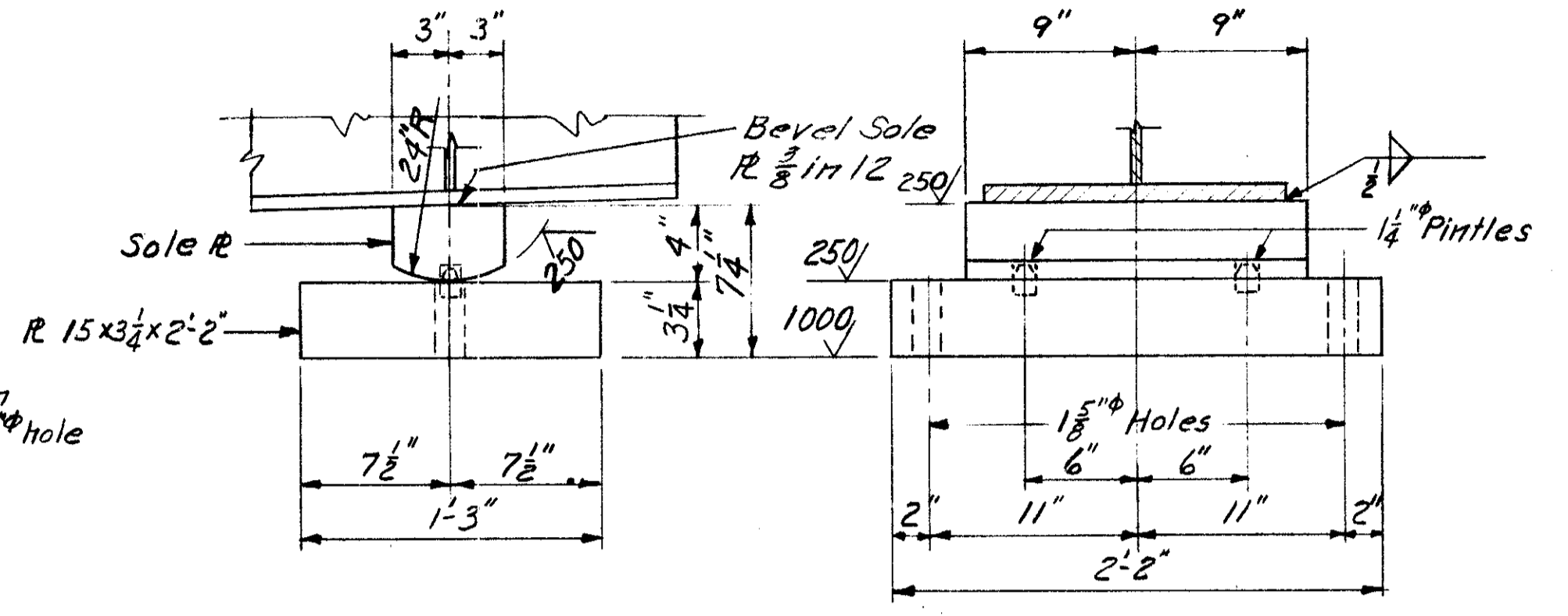
STRUCTURAL STEEL DETAILS

BRIDGE NO. HAM-71-0207

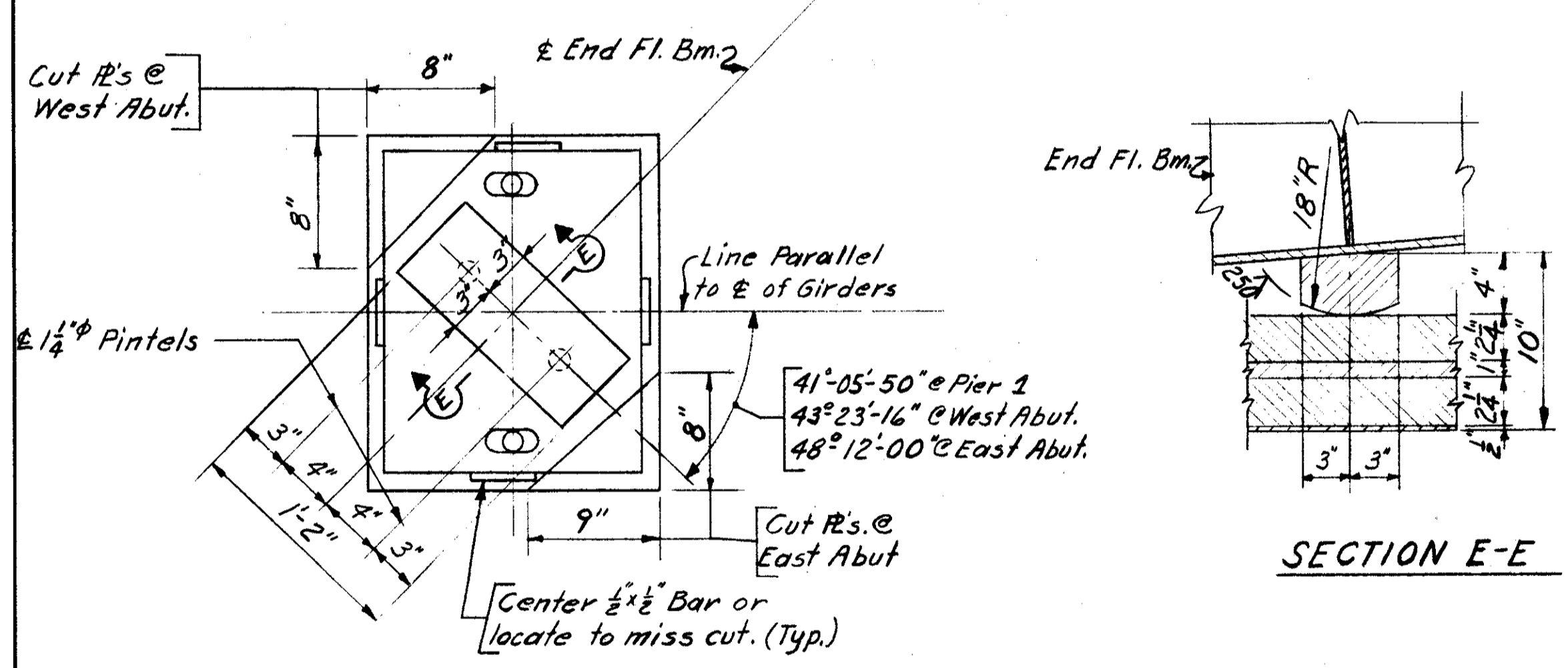
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
KFB			KFB	JHO	
			1-7-65	5/22/65	



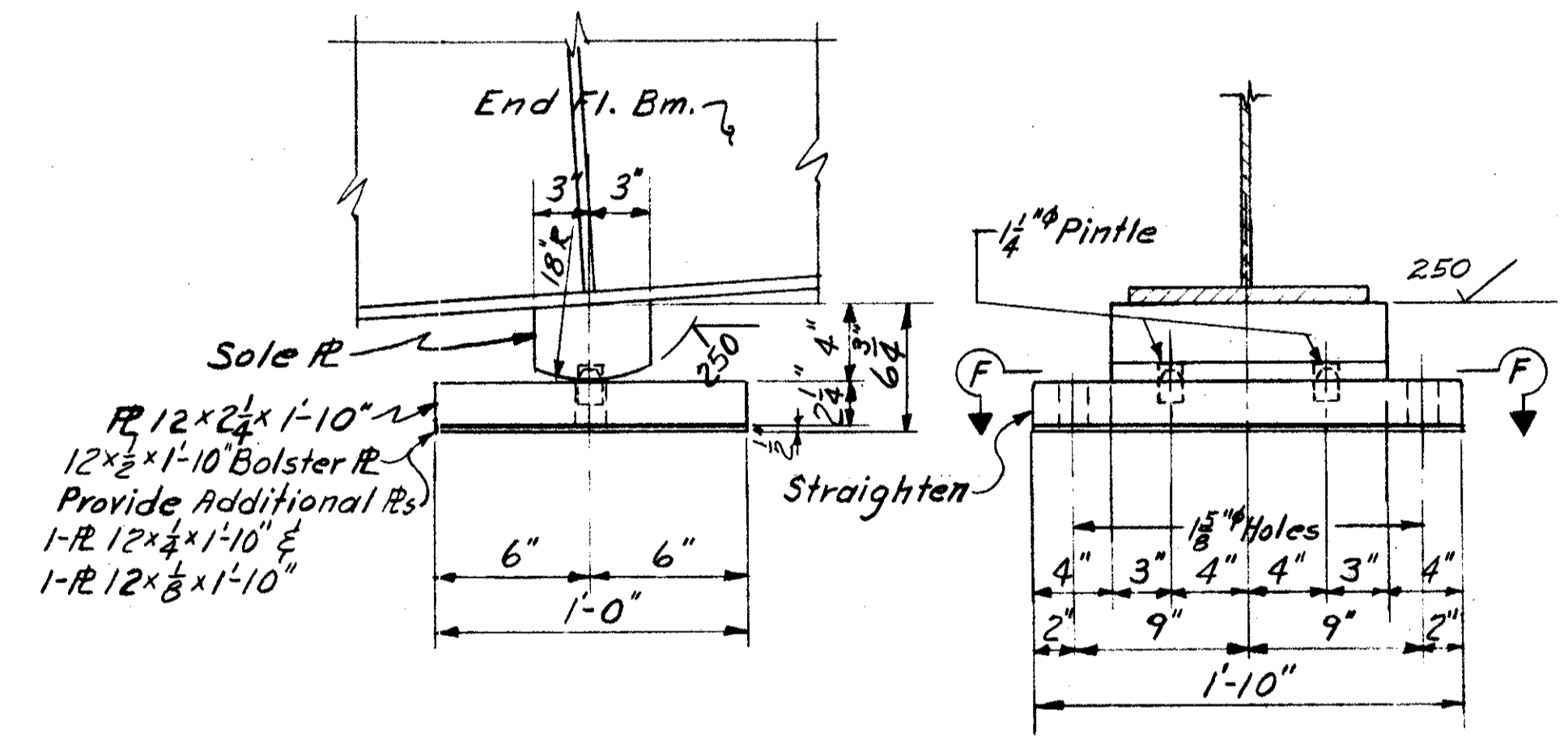
EXPANSION BEARING
(@ West Abut. Span 1)
(2- Req'd.)



FIXED BEARING
(@ Pier 1 Span 1)
(2- Req'd.)



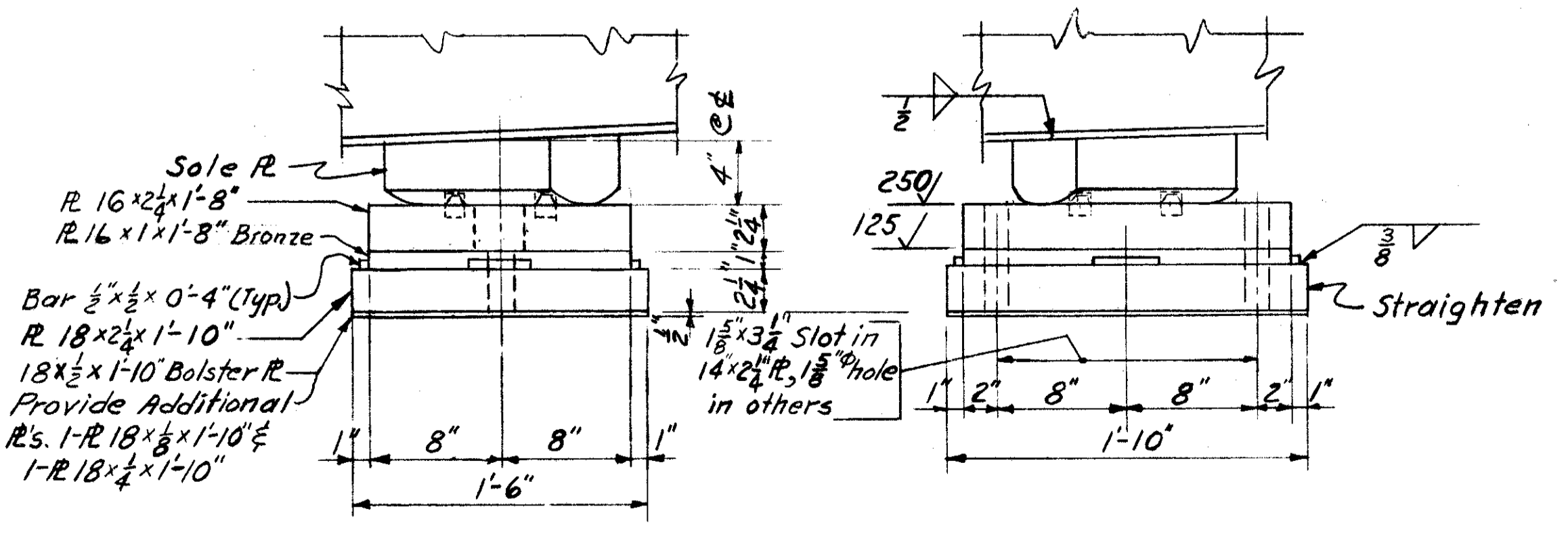
SECTION E-E



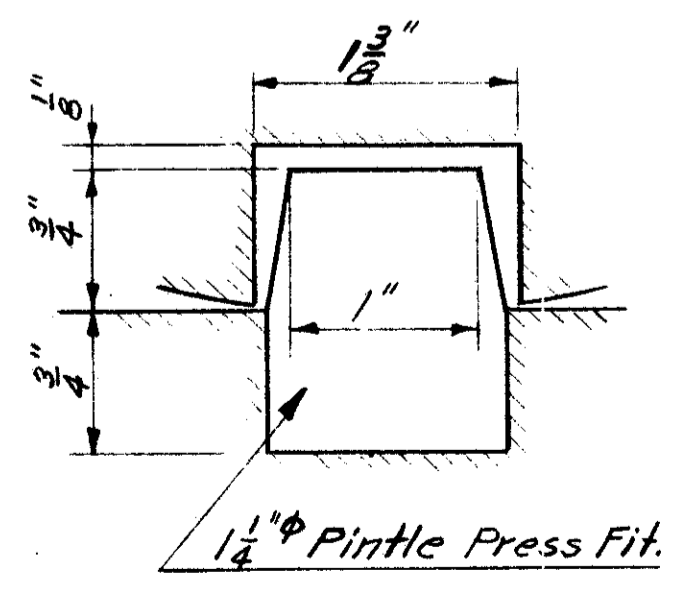
FIXED BEARING
(Under End Floor Bm. @ Pier 1 Span 1)
(1- Req'd.)

BEARING NOTES:
Material- Structural steel A36 or equal unless noted.
Steel castings A27 Grade 65-35, fully annealed
Bronze Plate as per A.S.T.M. Specifications B-22 Class-B with Trepanned Graphite Inserts.

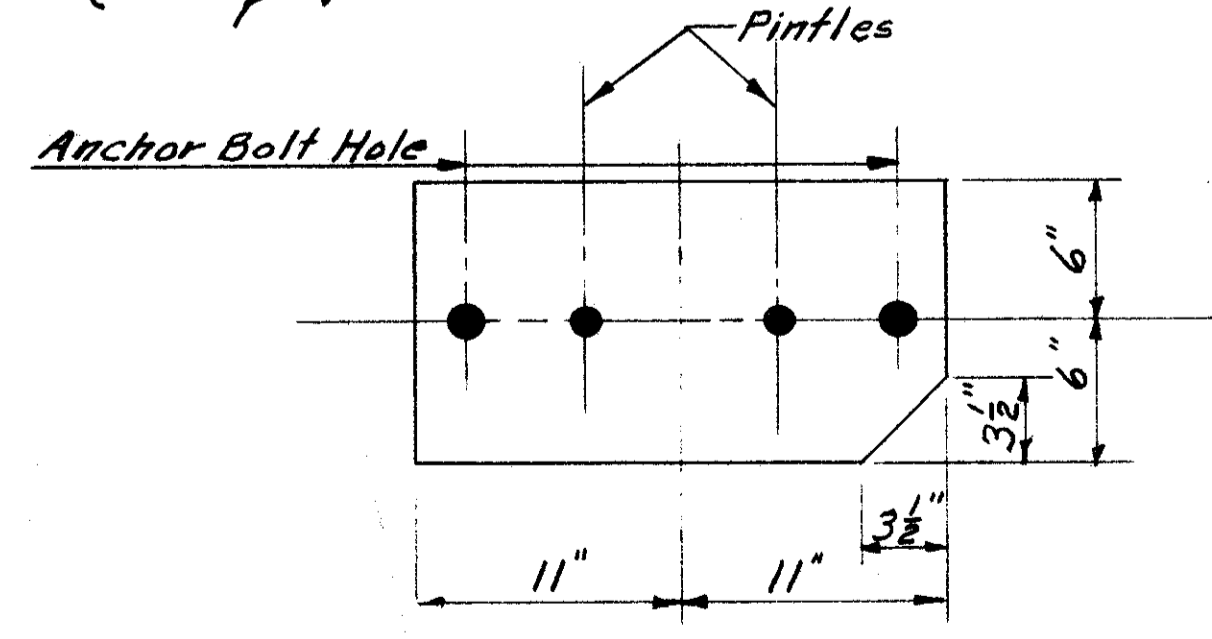
Pins may be either A235 Grade C1 annealed forgings or A108 Grade 1016 to 1030 cold finished shafting.
Recessed pin nuts may be either pressed or cast steel.
Sheet lead B29 common desilverized grade.
Finish designations indicated thus √ refer to A.S.A. standard B46.1-1955
See Abutment and Pier drawing for anchor bolt details.



EXPANSION BEARINGS
(Under End Floor Bm. @ Pier 1 Span 2,
East Abut. Span 3 & West Abut. Span 1)
(3- Req'd.)



PINTLE DETAIL



SECTION F-F

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FEB 20 1965

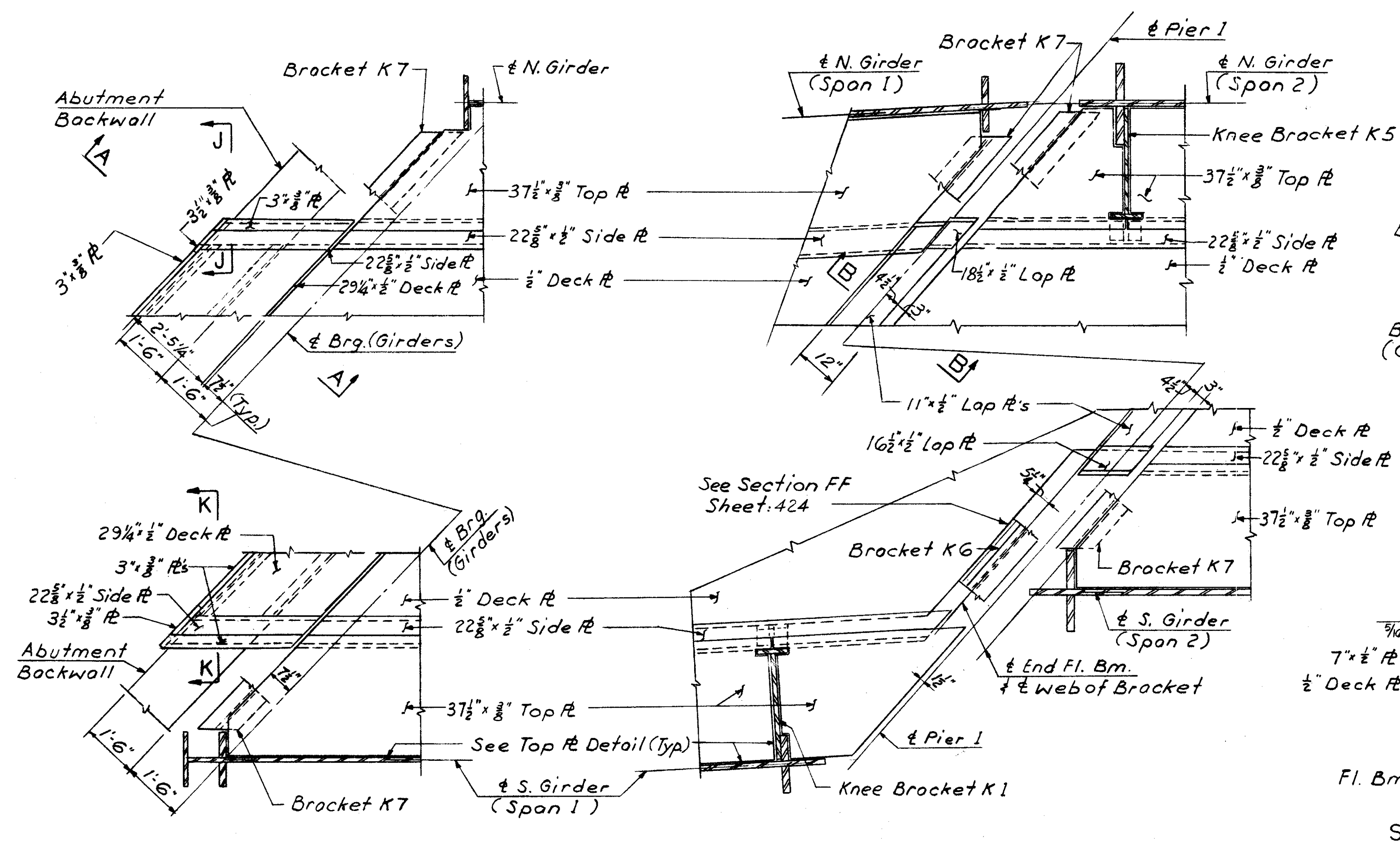
Work Sheets 425 & 426 together

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

STRUCTURAL STEEL DETAILS
BRIDGE NO. HAM - 71 - 0207

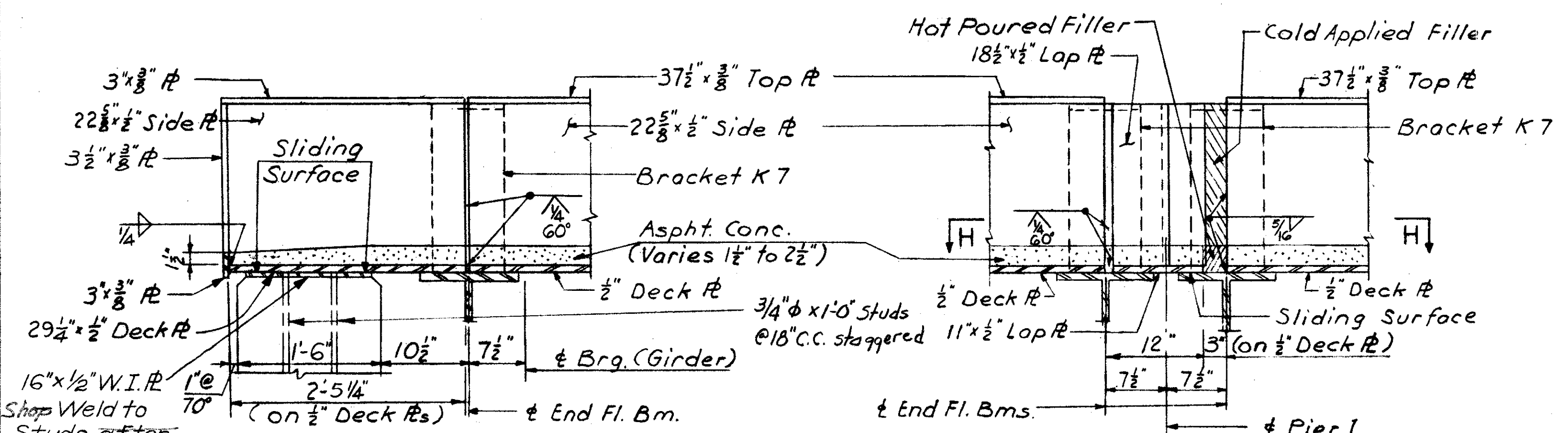
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
WFB	H.W.T.		WFB	1/10 5/22/65	

HAMILTON COUNTY
HAM-71-(1.56) (2.51)



PART DECK PLAN AT WEST ABUTMENT
(Deck Plan At East Abut. Similar By Rotation)

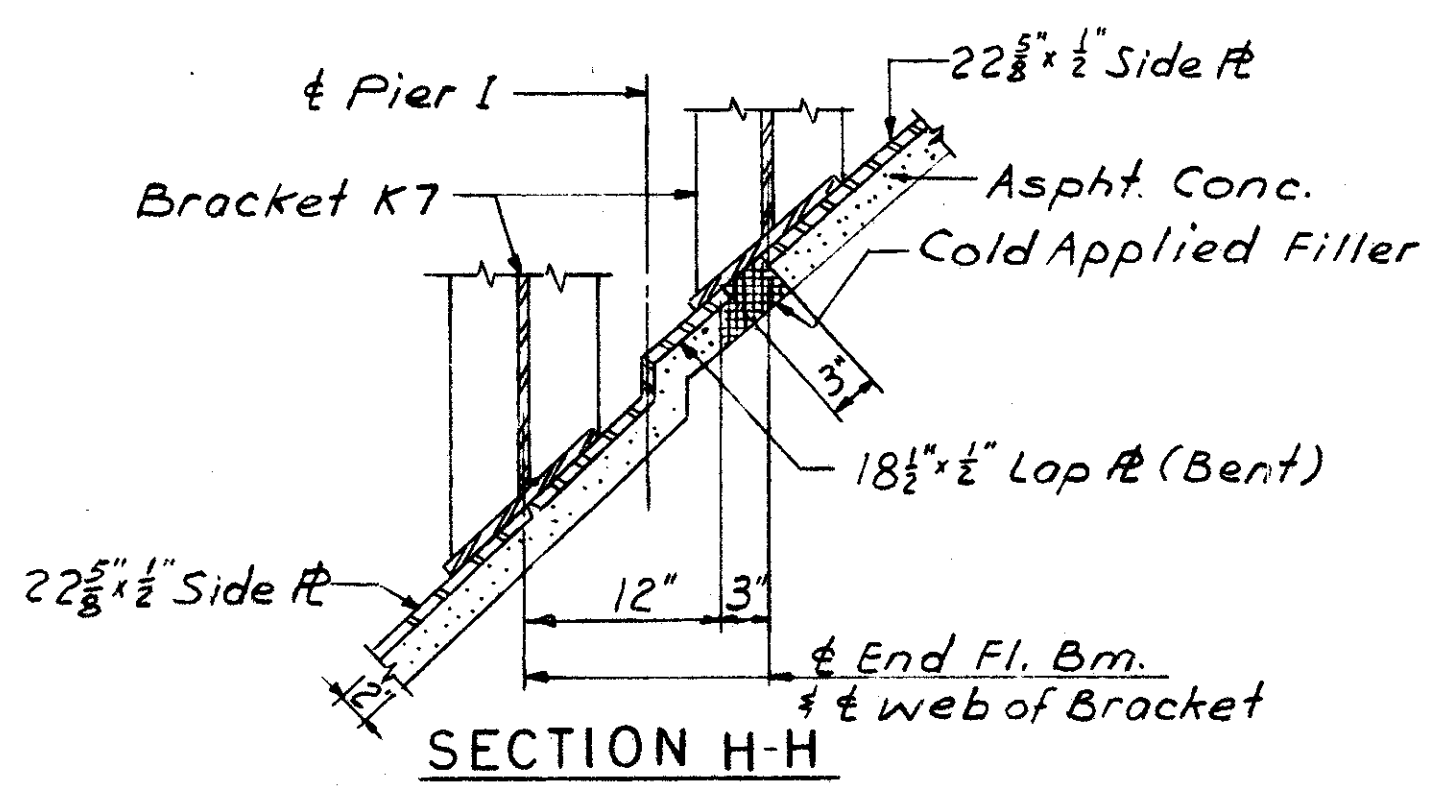
PART DECK PLAN AT PIER 1



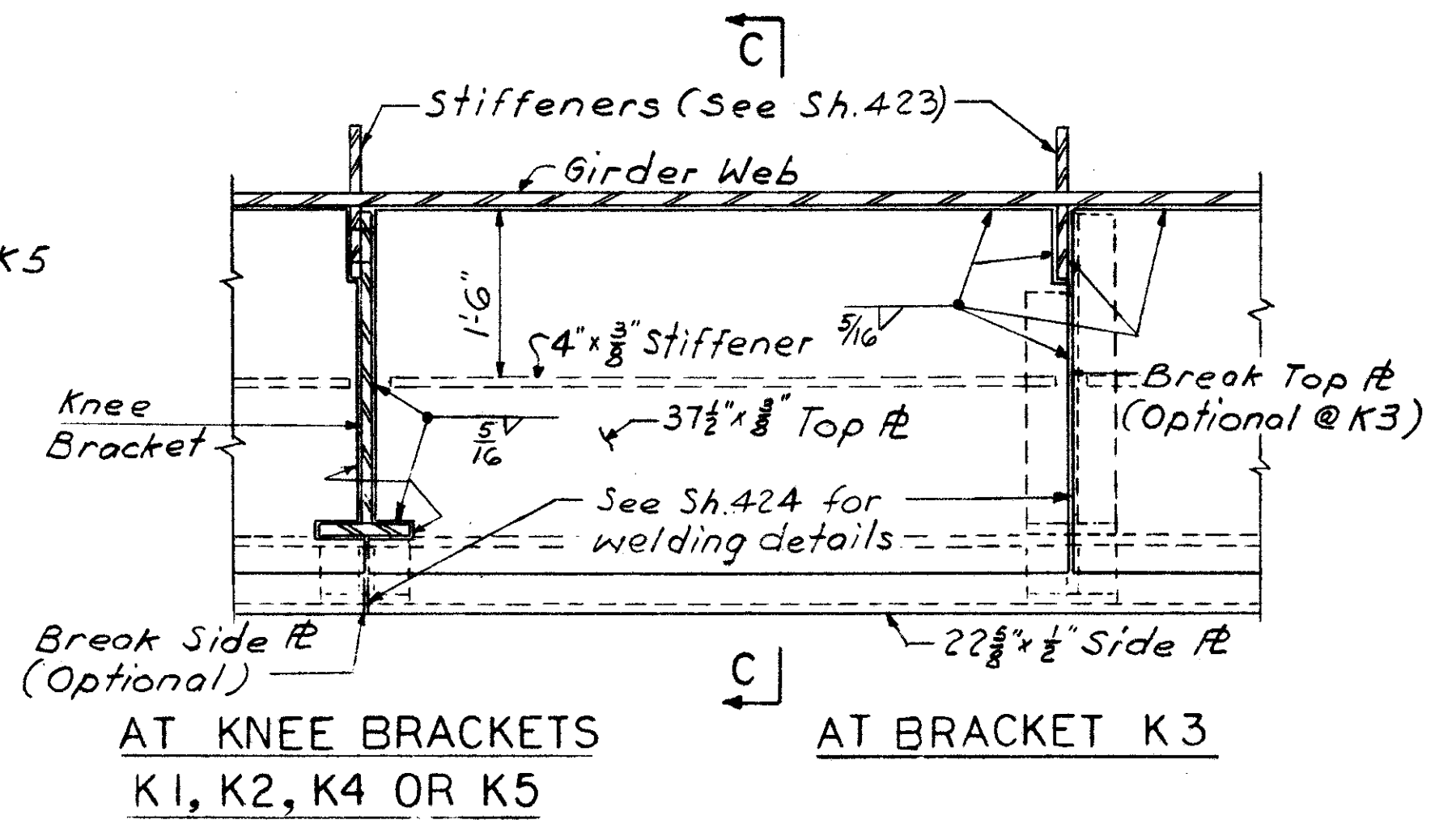
SECTION A-A

SECTION B-B

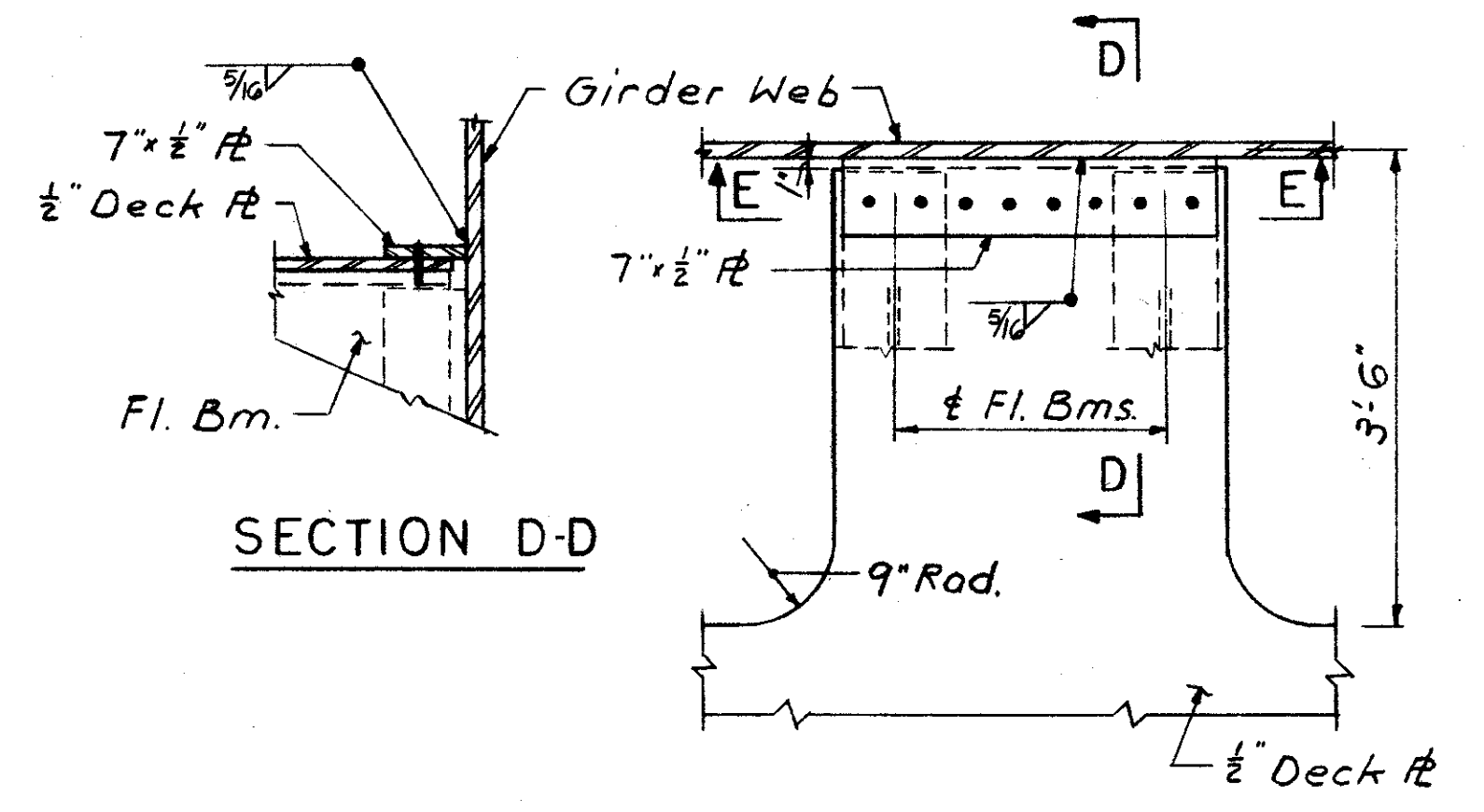
16"x1/2" W.I. Pl.
Shop Weld to
Studs after
Concrete is
set.
(Furnished under
item 5-107)



SECTION H-H

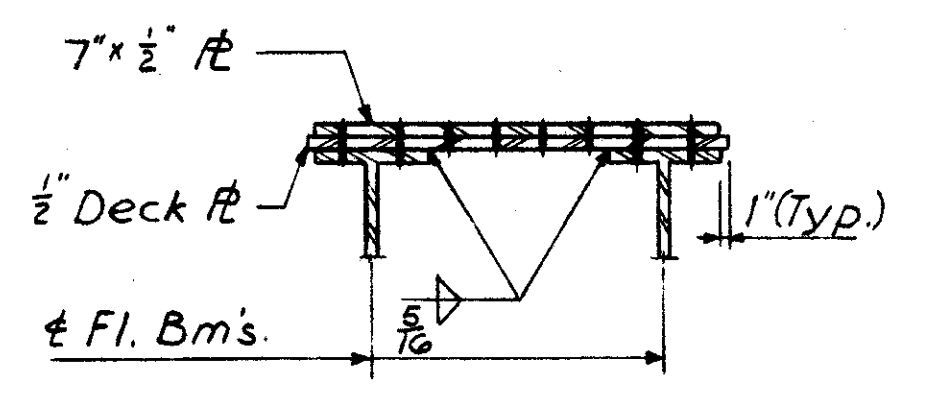


TOP PLATE DETAILS

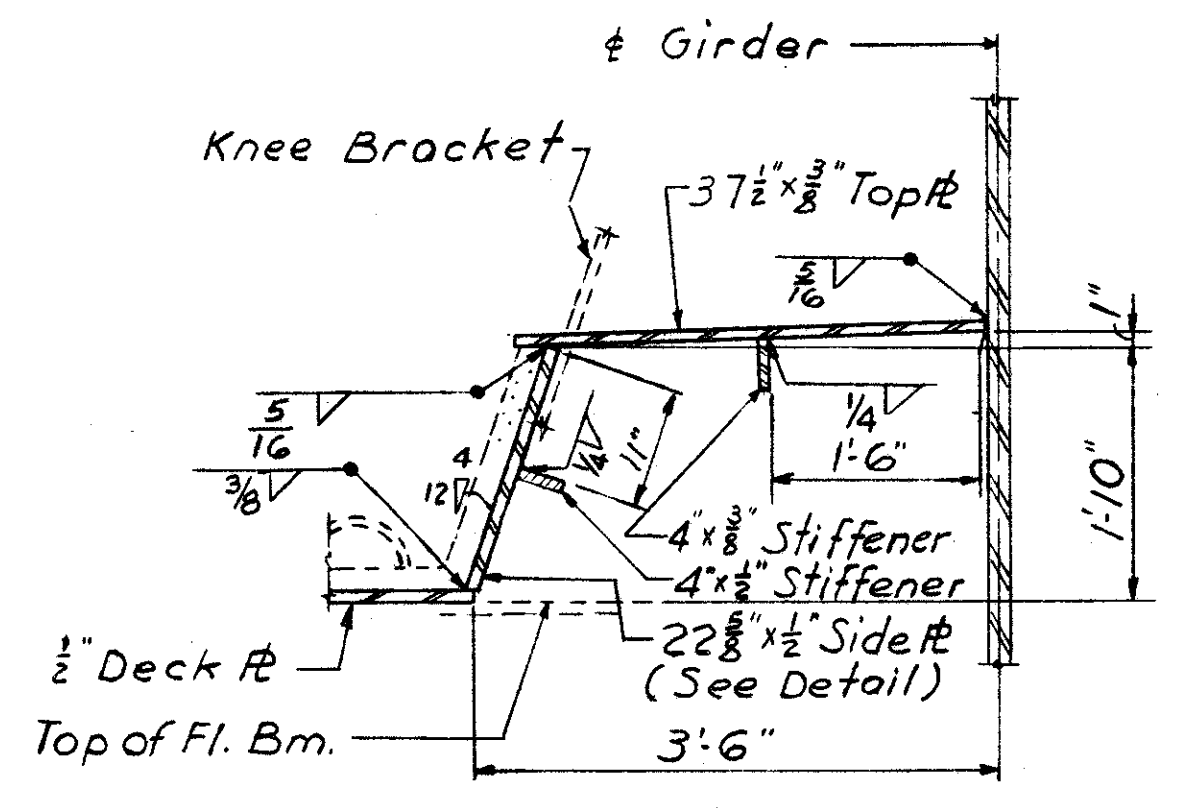


SECTION D-D

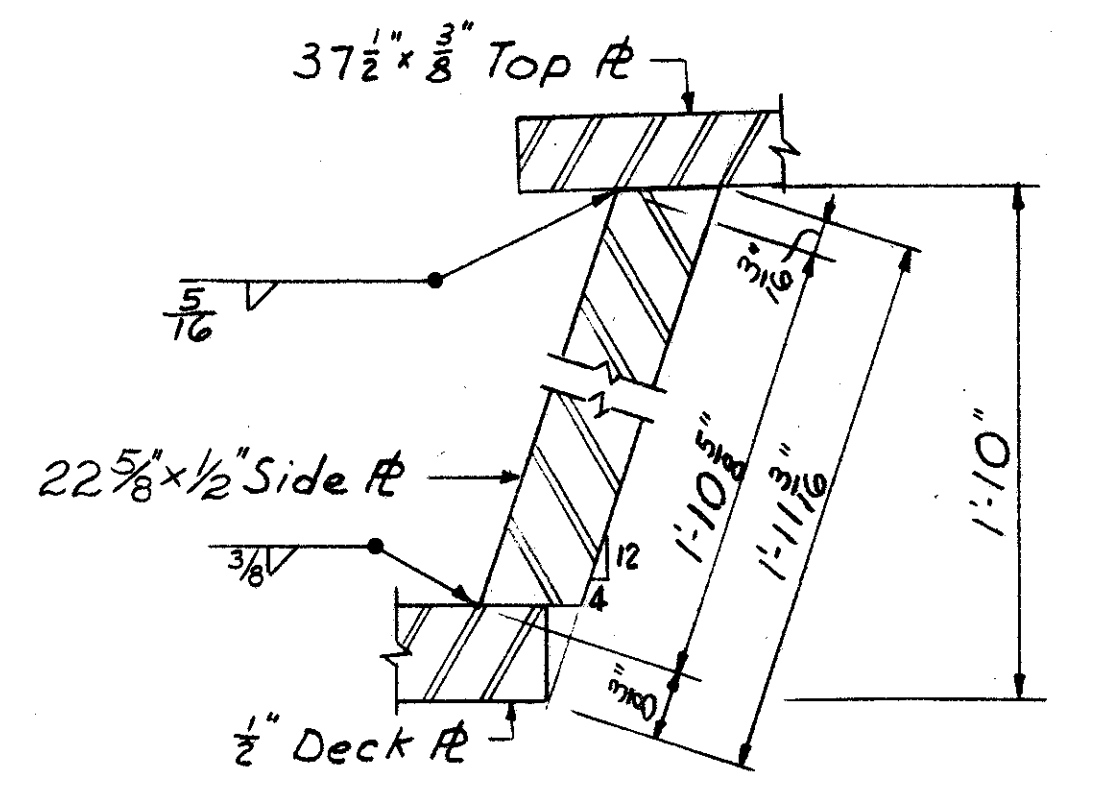
LATERAL PLATE CONN.
(For Location See Sheet 423)



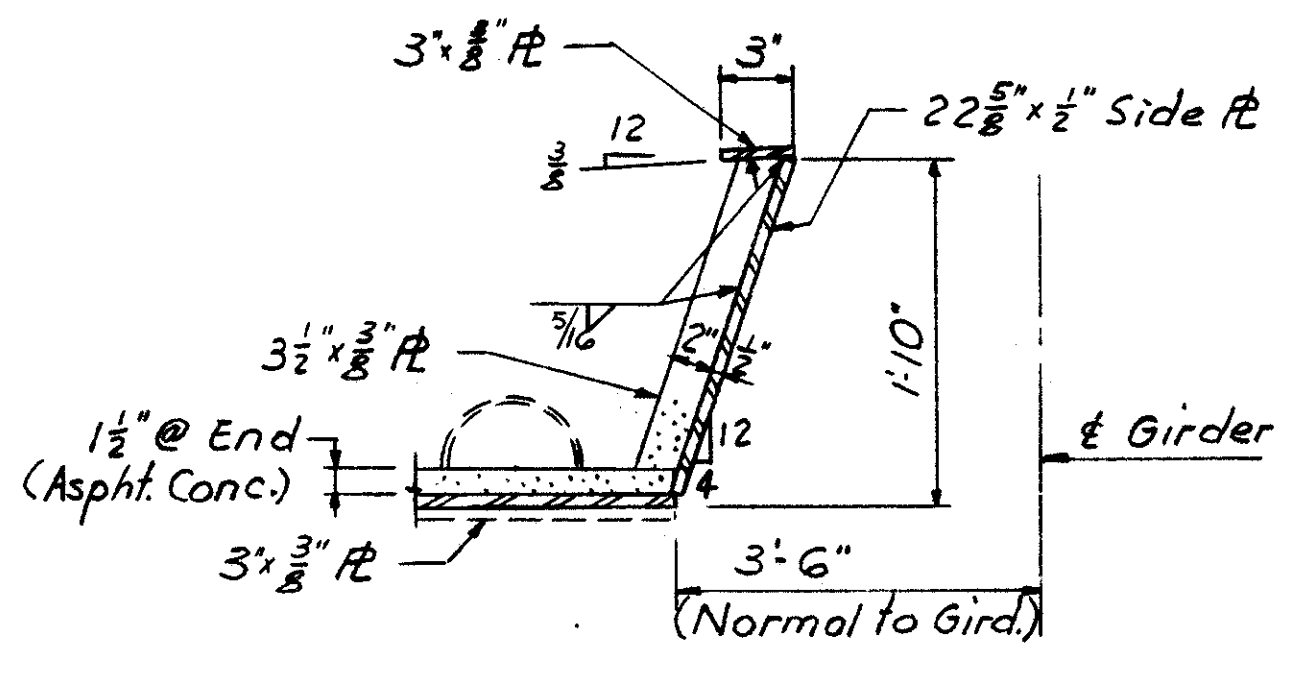
SECTION E-E



SECTION C-C



SIDE PLATE DETAIL



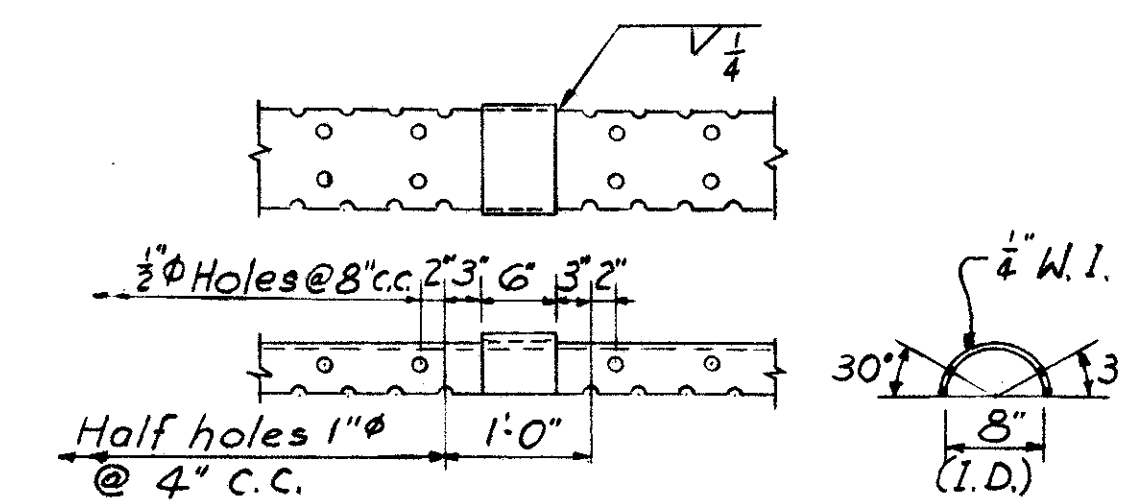
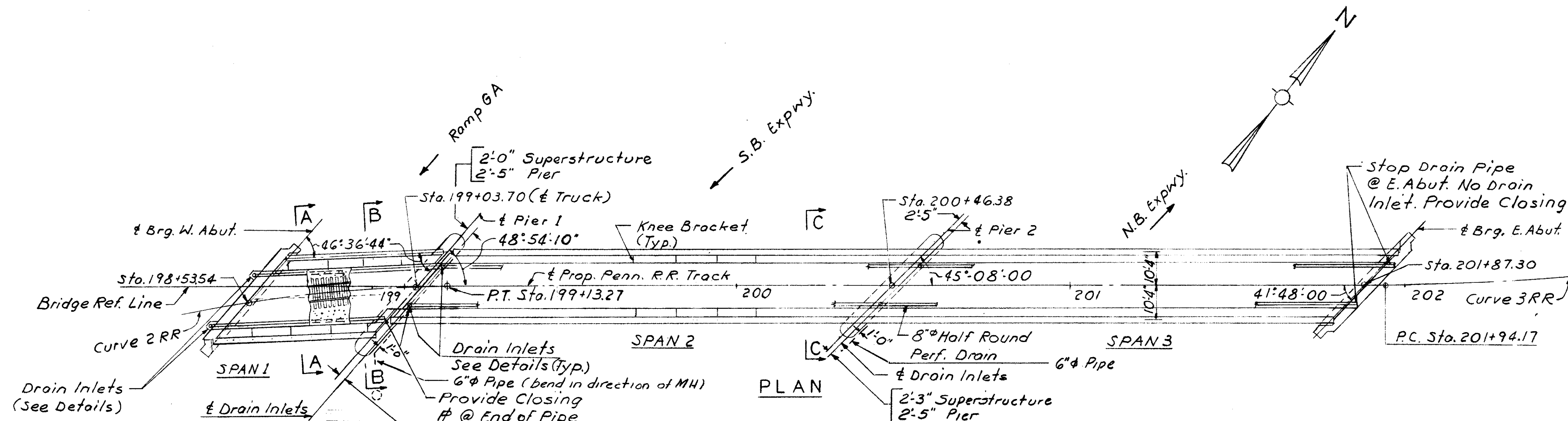
SECTION J-J
(SECTION K-K OPP. HAND)

Notes:
Deck plates, side plates and top plates shall be wrought iron.
Expansion Joint openings shown on details are for a normal temperature of 70° F.
Hot Poured Filler & Cold Applied Filler incidental to Asphalt Conc. Deck Protection.

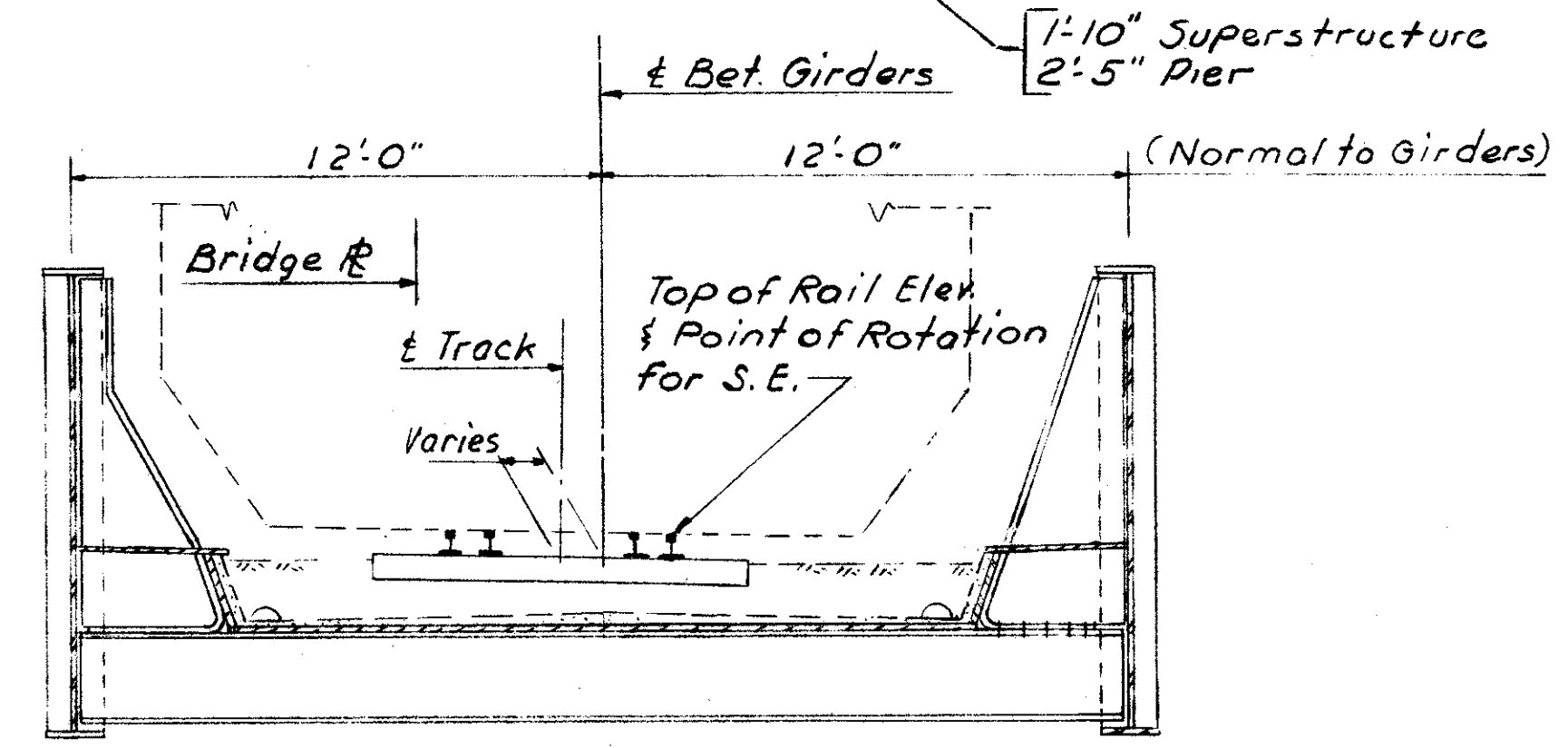
HAZLET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
DECK DETAILS					
BRIDGE NO. HAM-71-0207					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	S.U.		R.L.K.	JHO 5/22/65	1/23-65

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FEB 20 1965

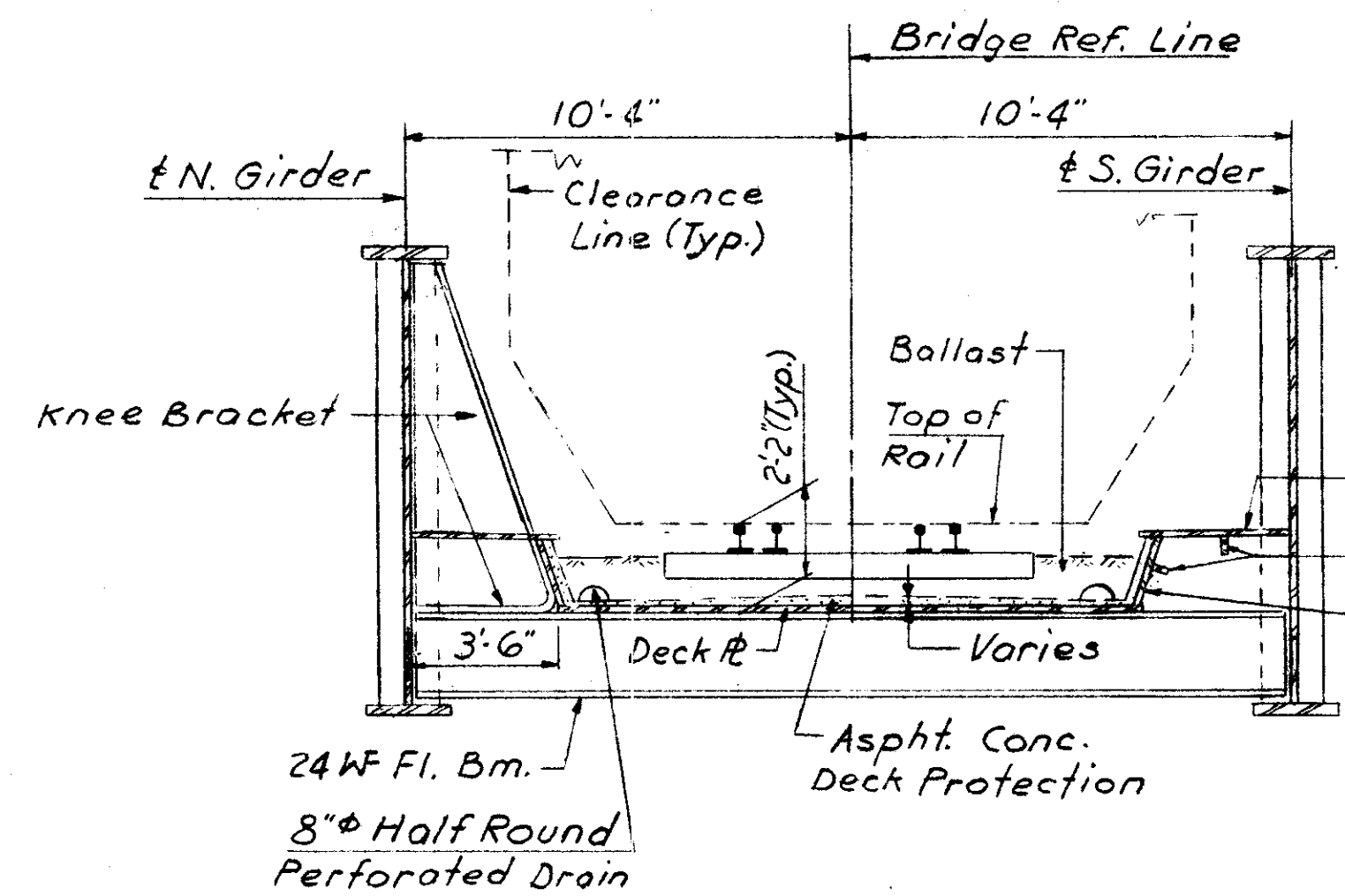
**HAMILTON COUNTY
HAM-71-(1.56) (2.51)**



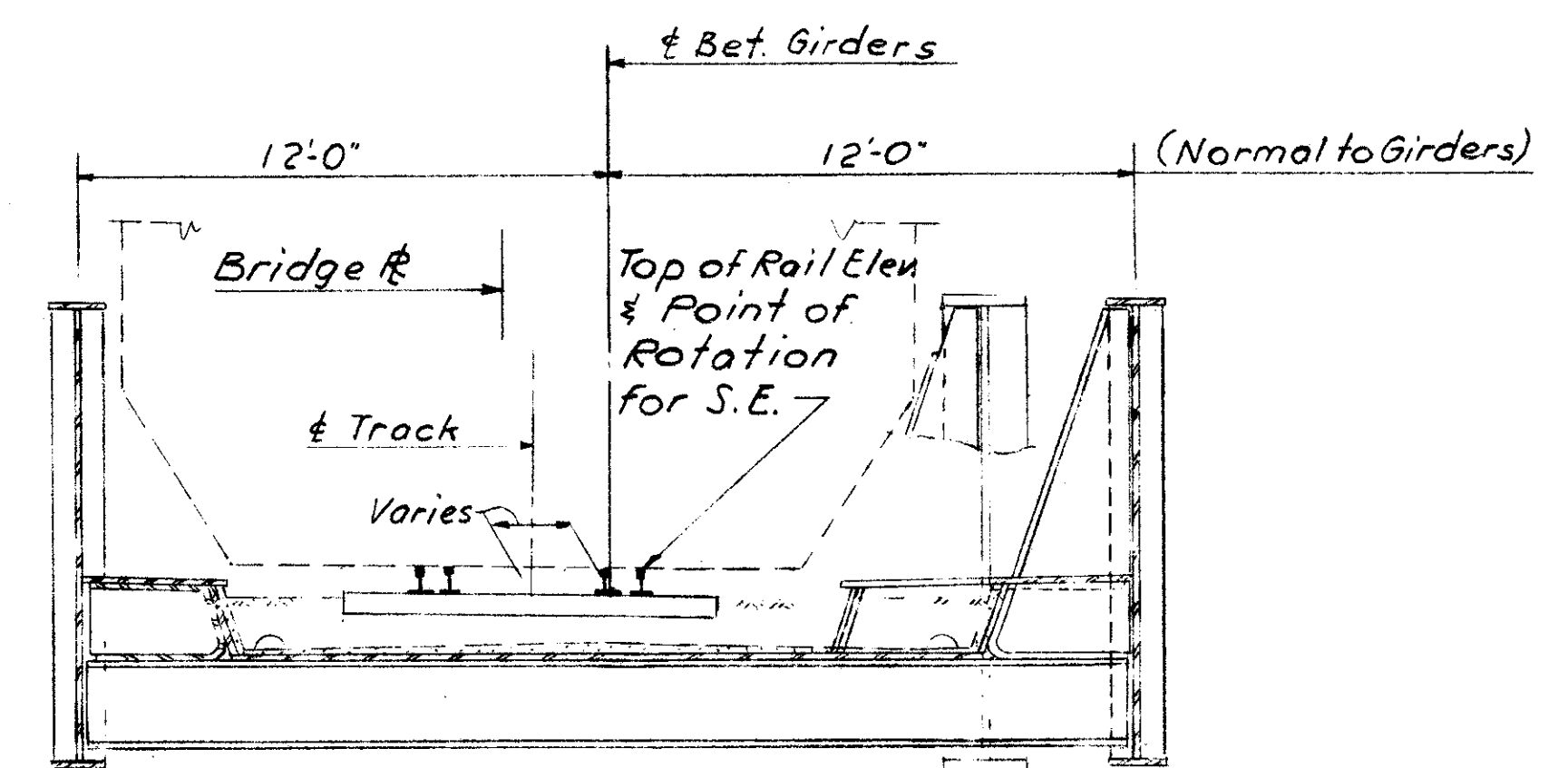
W.I. DRAINAGE PIPE DETAILS



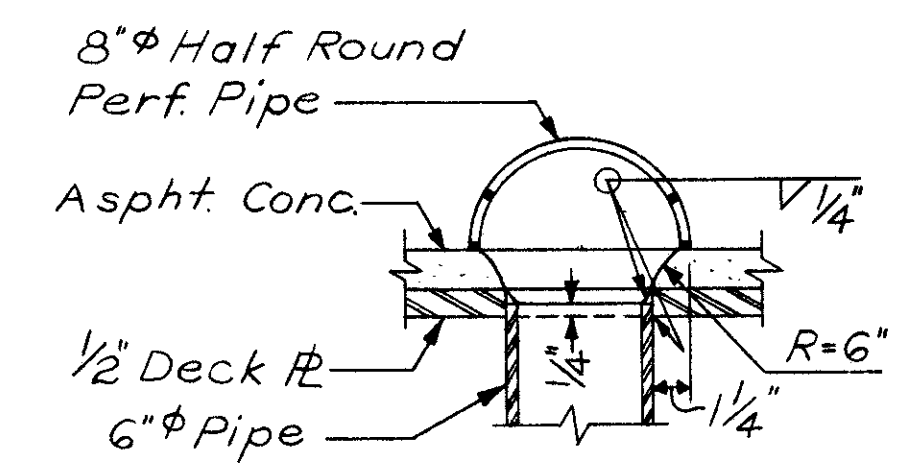
**SECTION A-A
(For Details not shown see Sect. C-C)**



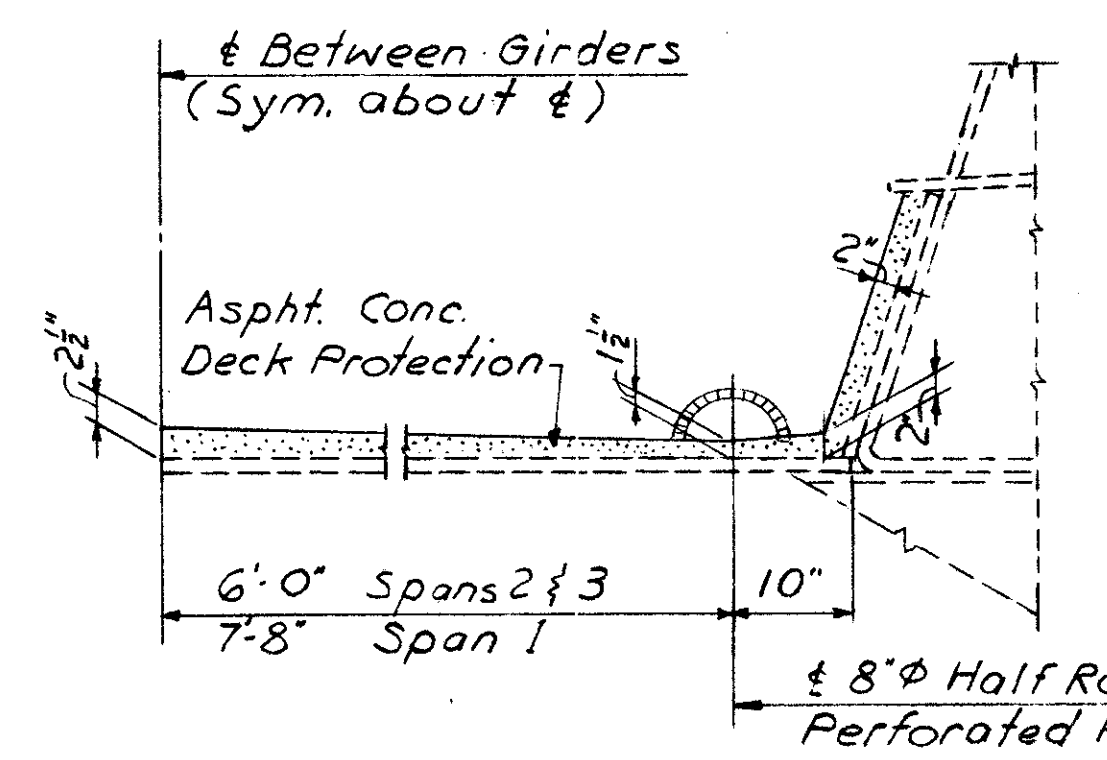
SECTION C-C



**SECTION B-B
(For Details not shown see Sect. C-C)**

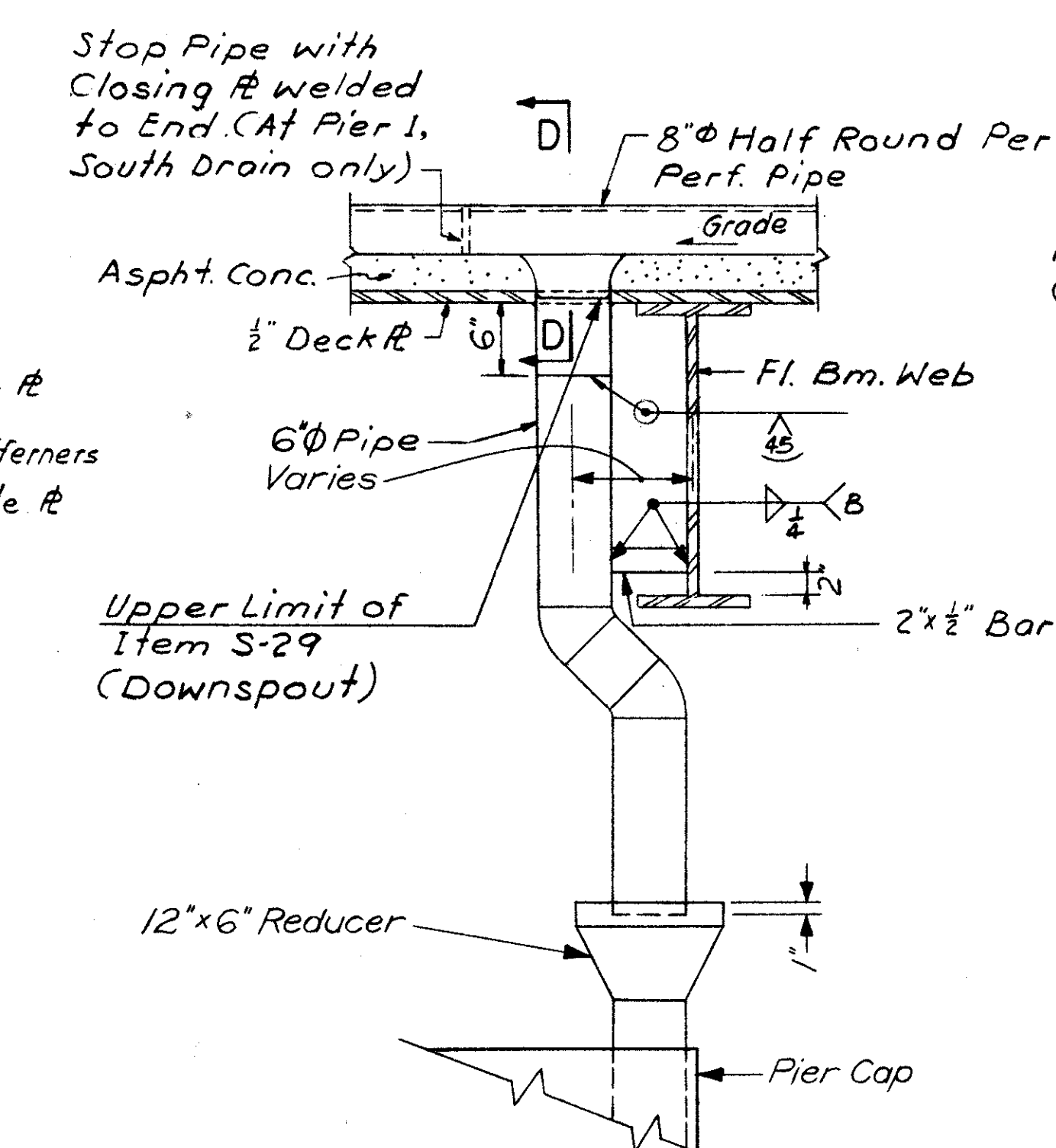


SECTION D-D



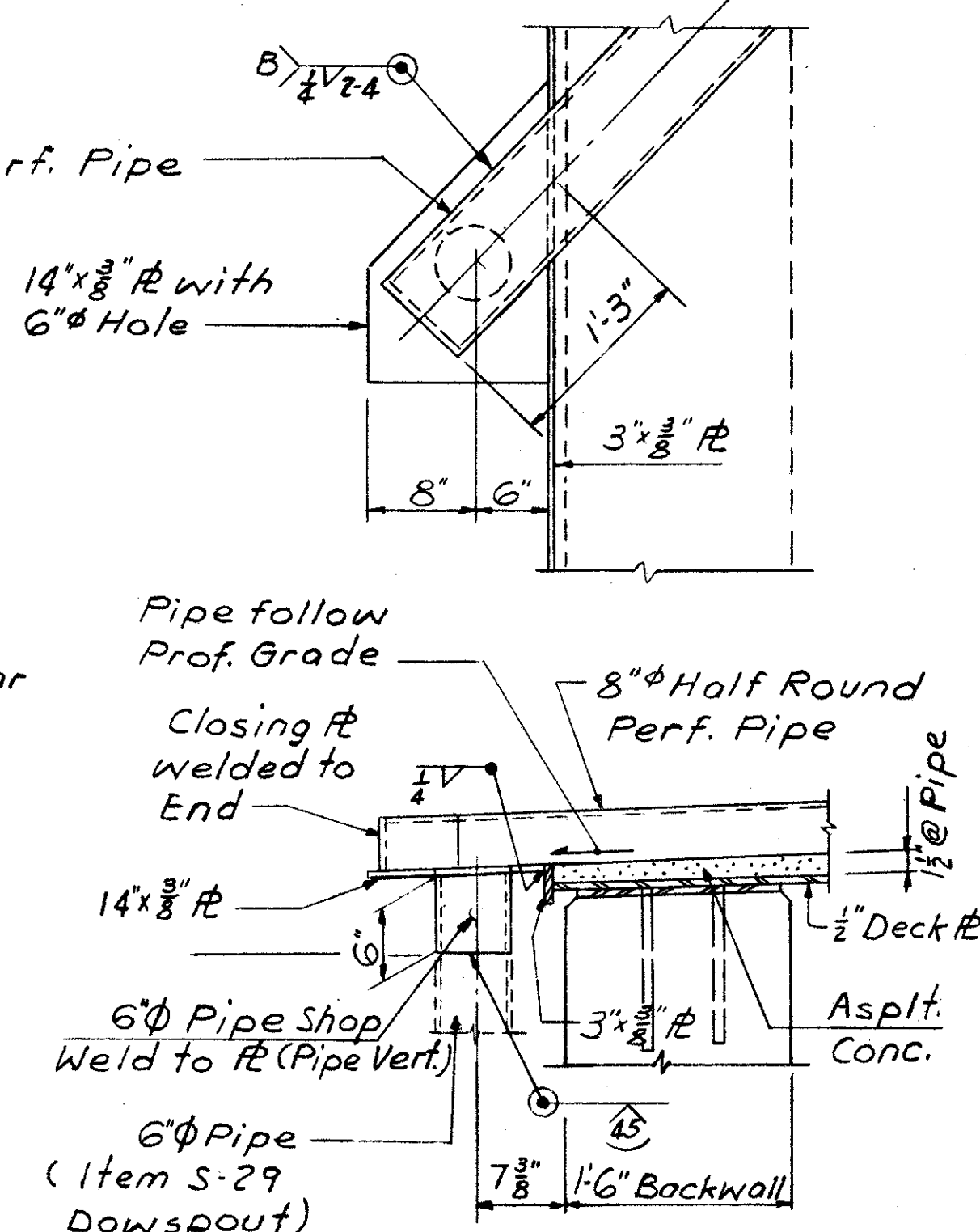
ASPHALT CONC. DECK PROTECTION

MICROFILMED
FEB 20 1985

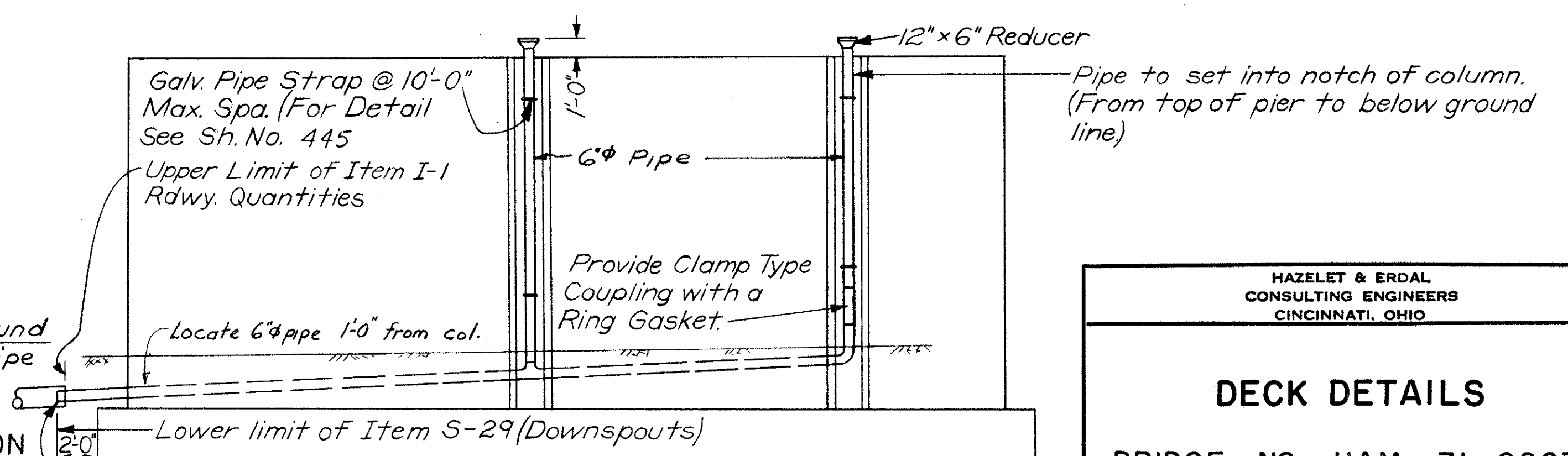


**DRAIN INLET AT PIERS
(Downspout at Piers 1 & 2)**

MICROFILMED
FEB 20 1985



DRAIN INLET AT WEST ABUT.



**HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO**

DECK DETAILS

BRIDGE NO. HAM-71-0207

**ELEVATION OF PIERS NOS. 1 & 2
SHOWING LOCATION OF DOWNSPOUTS**

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	S.U.		R.L.K.	1-11-65	
				5/22/65	

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

WEST ABUTMENT				
Mark	Type	No.Reqd.	Length	Weight
A501	Str.	10	17'-6"	183
A502		12	18'-0"	225
A503		10	5'-6"	57
A504		12	6'-0"	75
A505		24	19'-3"	482
A506		26	23'-9"	644
A507		10	14'-9"	154
A508	Str.	10	20'-3"	211
A509	Str.	5	7'-10"	41
A510		5	7'-1"	37
A511		5	6'-8"	35
A512		5	6'-0"	31
A513		1	9'-2"	10
A514		1	7'-10"	8
A515		1	8'-2"	9
A516		1	8'-10"	7
A517		1	12'-6"	13
A518		1	12'-1"	13
A519		1	11'-5"	12
A520	Str.	1	11'-4"	12
A521		40	6'-11"	289
A522		2	6'-6"	14
A523		2	6'-0"	13
A524		1	5'-6"	6
A525	20	1	4'-11"	5
A526	20	1	3'-8"	4
A527	14	3	5'-9"	18
A528	14	1	6'-0"	6
A601	Str.	36	22'-9"	1230
A602	20	12	7'-6"	135
A603	8	12	9'-1"	164
A604	20	13	8'-1"	158
A605	8	13	9'-7"	187
A606	20	1	8'-3"	12
A607	Str.	13	8'-3"	161
A608	20	4	9'-11"	60
A609	Str.	1	6'-9"	10
A610	Str.	15	8'-6"	192
A611	20	9	10'-9"	145
A901	Str.	86	15'-6"	4532
A1101	44	56	20'-1"	5975
A1102	18	87	7'-7"	3505
A1103	Str.	43	9'-0"	2056
A1104	Str.	20	17'-6"	1860
A1105	Str.	24	18'-3"	2327
H501*	Str.	2	3'-10"	
H502*	Str.	2	3'-2"	
Total				25,323

EAST ABUTMENT				
Mark	Type	No.Reqd.	Length	Weight
A551	Str.	12	15'-9"	197
A552		10	15'-0"	156
A553		11	5'-3"	60
A554		10	4'-6"	47
A555		22	22'-6"	516
A556		22	18'-6"	425
A557	Str.	12	18'-3"	228
A558	14	7	5'-9"	42
A559	52	6	7'-7"	47
A560	52	6	7'-0"	44
A561	Str.	12	14'-9"	185
A562	52	6	6'-10"	43
A563		6	6'-5"	40
A564		1	7'-10"	8
A565		1	8'-7"	9
A566		1	8'-11"	7
A567		1	13'-2"	14
A568		1	13'-6"	14
A569		1	12'-7"	13
A570	52	1	12'-10"	13
A571		33	6'-11"	238
A573	Str.	3	4'-8"	15
A574	52	1	9'-1"	10
A575		2	6'-6"	14
A576		2	6'-0"	13
A577		1	5'-5"	6
A651	Str.	18	41'-6"	1122
A652	20	11	10'-3"	169
A653	8	21	10'-10"	342
A654	20	10	10'-4"	155
A658	Str.	13	10'-0"	195
A659	Str.	16	9'-6"	228
A660	Str.	1	8'-0"	12
A661	20	1	10'-9"	16
A662	20	5	12'-7"	95
A663	20	12	11'-10"	213
A664		5	11'-10"	89
A951	Str.	82	15'-6"	4322
A1051	18	84	7'-4"	2650
A1052	Str.	42	10'-0"	1807
A1053	Str.	23	15'-9"	1559
A1054	Str.	19	15'-3"	1247
A1151	44	52	20'-1"	5549
H553*	52	2	7'-8"	
H554*	52	2	7'-8"	
Total				22,174

PIER 1				
Mark	Type	No.Reqd.	Length	Weight
P511	Str.	6	9'-9"	61
P512	Str.	10	7'-9"	81
P513	52	2	13'-10"	29
P514	Str.	1	14'-0"	15
P515	Str.	1	15'-9"	16
P516	18	2	21'-8"	45
P517	6	24	11'-3"	282
P518	1	9	13'-5"	126
P519		12	14'-1"	176
P520		16	7'-11"	132
P521		3	13'-4"	42
P522		2	13'-2"	27
P523		2	12'-7"	26
P524		1	12'-0"	13
P525		1	11'-5"	12
P526		1	10'-10"	12
P527	Str.	29	15'-3"	461
P528	Str.	29	15'-9"	476
P529	Str.	1	17'-3"	18
P530	Str.	9	22'-4"	210
P531	Str.	9	20'-3"	190
P534	58	9	22'-9"	213
P535	58	9	20'-8"	194
P536	58	2	8'-2"	17
P611	44	16	25'-5"	611
P612	43	54	12'-10"	1041
P613	18	98	5'-10"	859
P711	Str.	2	23'-0"	94
P712	Str.	2	20'-3"	83
P713	Str.	6	22'-3"	273
P714	Str.	6	25'-0"	307
P715	58	2	20'-6"	84
P716	58	2	23'-3"	95
Total				6,321

PIER 2				
Mark	Type	No.Reqd.	Length	Weight
P501	Str.	7	19'-10"	145
P502	Str.	7	17'-9"	130
P503	Str.	6	15'-8"	98
P504	Str.	46	15'-0"	720
P505	Str.	4	9'-3"	39
P506	6	19	11'-3"	223
P507	1	5	8'-6"	44
P508	1	35	9'-3"	338
P509	Str.	1	7'-6"	8
P510	58	7	18'-2"	133
P532	58	7	20'-3"	148
P533	52	1	7'-9"	8
P601	44	20	23'-7"	708
P602	18	88	5'-10"	771
P801	43	59	16'-8"	2626
P1001	Str.	6	21'-3"	549
P1002	Str.	2	23'-9"	204
P1003	Str.	6	18'-0"	465
P1004	Str.	2	20'-0"	172
P1005	58	2	17'-9"	153
P1006	58	2	21'-0"	181
Total				7,863

REPLACEMENT BARS			
Mark	Type	No.Reqd.	Length
Re 501	Str.	1	5'-7"
Re 601		1	5'-11"
Re 701		1	6'-3"
Re 801		1	6'-6"
Re 901		1	6'-10"
Re 1001		1	7'-3"
Re 1101	Str.	2	7'-6"

Type	Mark	a
43	P612	11'-6"
	P801	14'-6"

TYPE 43

Type	Mark	a
44	A1101	18'-6"
	A1151	18'-6"
	P601	22'-11"

TYPE 44

Type	Mark	a	b	Radius
6	P506	8'-1"	1'-7"	2'-6"
	P517	8'-1"	1'-7"	2'-6"

TYPE 6

Type	Mark	a	b	c	d	e
52	A509	1'-8"	3'-18"	2'-0"	4'-4"	3'-0 1/2"
	A510	2'-10"	1'-5 1/2"	2'-3"	2'-2 1/2"	1'-6 1/2"
	A511	0'-9"	2'-10 1/2"	2'-0"	4'-2"	3'-0 1/2"
	A512	1'-9"	1'-5 1/2"	2'-5"	2'-1"	1'-6 1/2"
	A513	1'-8"	3'-1 1/2"	3'-4"	4'-4"	3'-0 1/2"
	A514	2'-10"	1'-5 1/2"	3'-0"	2'-2 1/2"	1'-6 1/2"
	A515	1'-8"	3'-1 1/2"	2'-4"	4'-4"	3'-0 1/2"
	A516	2'-10"	1'-5 1/2"	2'-0"	2'-2 1/2"	1'-6 1/2"
	A517	0'-9"	2'-10 1/2"	2'-0"	4'-2"	3'-0 1/2"
	A518	1'-9"	1'-5 1/2"	8'-6"	2'-1"	1'-6 1/2"
	A519	0'-9"	2'-10 1/2"	6'-9"	4'-2"	3'-0 1/2"
	A520	1'-9"	1'-5 1/2"	7'-9"	2'-1"	1'-6 1/2"
	A559	1'-10"	2'-7"	2'-0"	3'-1 1/2"	3'-0 1/2"
	A560	2'-10"	1'-4"	2'-5"	2'-0"	1'-6 1/2"
	A562	0'-6"	3'-4 1/2"	2'-0"	4'-6"	3'-0 1/2"
	A563	1'-11"	1'-8"	2'-5"	2'-3"	1'-6 1/2"
	A564	2'-10"	1'-4"	3'-3"	2'-0"	1'-6 1/2"
	A565	1'-10"	2'-7"	3'-0"	3'-1 1/2"	3'-0 1/2"
	A566	2'-10"	1'-4"	2'-4"	2'-0"	1'-6 1/2"
	A567	0'-6"	3'-4 1/2"	8'-4"	4'-6"	3'-0 1/2"
A568	1'-11"	1'-8"	9'-6"	2'-3"	1'-6 1/2"	
A569	0'-6"	3'-4 1/2"	7'-9"	4'-6"	3'-0 1/2"	
A570	1'-11"	1'-8"	8'-10"	2'-3"	1'-6 1/2"	
A574	1'-10"	2'-7"	3'-0"	3'-1 1/2"	3'-0 1/2"	
H553	3'-1 1/2"	3'-1 1/2"	0'-6 1/2"	4'-2"	2'-9 1/2"	
H554	3'-4"	3'-1 1/2"	0'-4"	4'-2"	2'-9 1/2"	
P513	11'-1"	0'-4 1/2"	2'-2"	0'-10"	0'-9"	
P533	1'-4"	0'-4 1/2"	5'-10 1/2"	0'-10"	0'-9"	

TYPE 52

Type	Mark	a	b	c	d
20	A525	2'-4"	1'-6"	3'-5"	2'-5 1/2"
	A526	0'-7 1/2"	2'-7"	0'-10 1/2"	0'-7 1/2"
	A602	2'-3 1/2"	1'-7"	3'-3"	2'-3 1/2"
	A604	2'-3 1/2"	4'-10"	3'-3"	2'-3 1/2"
	A606	2'-3 1/2"	5'-0"	3'-3"	2'-3 1/2"
	A608	2'-3 1/2"	6'-8"	3'-3"	2'-3 1/2"
	A611	2'-3 1/2"	7'-6"	3'-3"	2'-3 1/2"
	A652	2'-3 1/2"	7'-0"	3'-3"	2'-3 1/2"
	A654	2'-3 1/2"	7'-1"	3'-3"	2'-3 1/2"
	A661	2'-3 1/2"	7'-6"	3'-3"	2'-3 1/2"
	A662	2'-3 1/2"	9'-4"	3'-3"	2'-3 1/2"
	A663	2'-3 1/2"	8'-7"	3'-3"	2'-3 1/2"
	H551	1'-4 1/2"	2'-7 1/2"	1'-10 1/2"	1'-2 1/2"
	H552	1'-2"	2'-4"	1'-6 1/2"	1'-0 1/2"
	A603	2'-9"	1'-2"	5'-6"	
	A605	2'-9"	1'-2"	6'-0"	
	A653	2'-6"	1'-2"	7'-6"	
	A527	0'-8"	2'-3"		
	A528	0'-11"	2'-3"		
	A558	0'-8"	2'-3"		

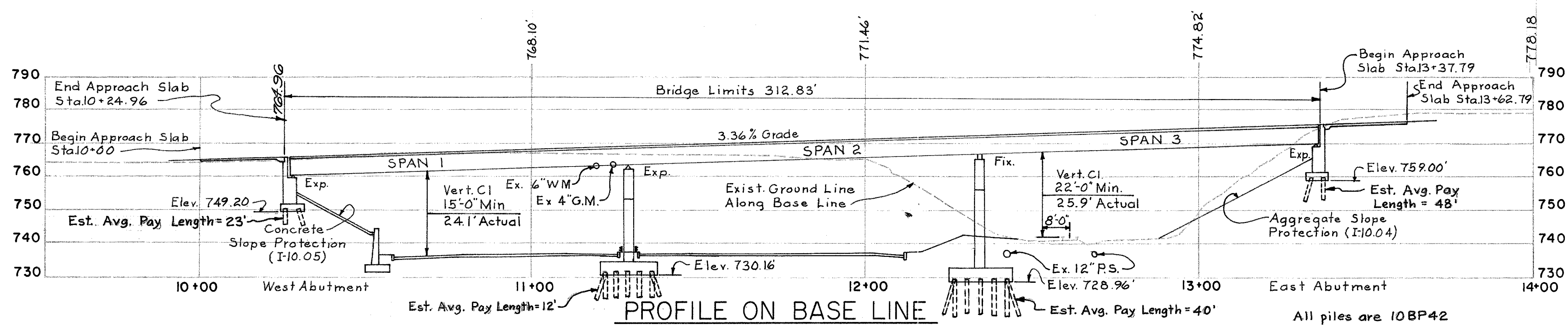
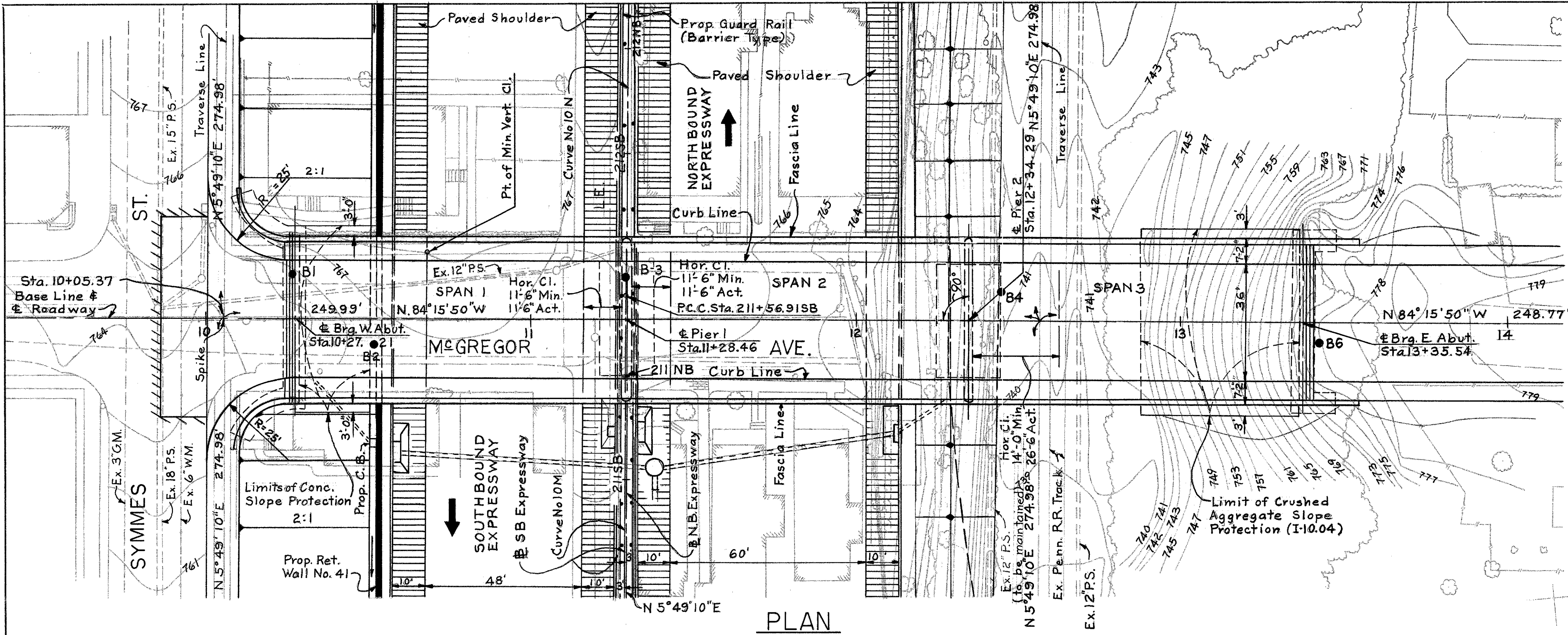
TYPES 20, 8, 14 & 18

Type	Mark	a	b
1	A521	4'-2"	1'-6"
	A522	3'-9"	1'-6"
	A523	3'-3"	1'-6"
	A524	2'-9"	1'-6"
	A571	4'-2"	1'-6"
	A575	3'-9"	1'-6"
	A576	3'-3"	1'-6"
	A577	2'-8"	1'-6"
	A664	2'-8"	4'-9"
	P507	4'-5"	2'-2"
	P508	5'-2"	2'-2"
	P518	5'-2"	4'-3"
	P519	5'-2"	4'-7"
	P520	5'-2"	1'-6"
	P521	4'-5"	4'-7"
	P522	4'-3 1/2"	4'-7"
	P523	3'-8 1/2"	4'-7"
	P524	3'-1 1/2"	4'-7"
	P525	2'-6 1/2"	4'-7"
	P526	1'-1 1/2"	4'-7"

TYPE 1

Type	Mark	a	b	c	d	e	f	g	h	i
58	P510	1'-8"	0'-4 1/2"	0'-10"	0'-4 1/2"	5'-11"	0'-9"	0'-9"	0'-10"	9'-5"
	P532	1'-8"	0'-4 1/2"	0'-10"	0'-4 1/2"	6'-8"	0'-9"	0'-9"	0'-10"	10'-9"
	P534	1'-8"	0'-4 1/2"	0'-10"	0'-4 1/2"	4'-0"	0'-9"	0'-9"	0'-10"	15'-1 1/2"
	P535	1'-8"	0'-4 1/2"	0'-10"	0'-4 1/2"	6'-8 1/2"	0'-9"	0'-9"	0'-10"	11'-1 1/2"
	P536	1'-8"	0'-4 1/2"	0'-10"	0'-4 1/2"	4'-1"	0'-9"	0'-9"	0'-10"	1'-3"
	P715	1'-8 1/2"	0'-4 1/2"	0'-10"	0'-4 1/2"	6'-8 1/2"	0'-9"	0'-9"	0'-10"	11'-1"
	P716	1'-8 1/2"	0'-4 1/2"	0'-10"	0'-4 1/2"					

HAMILTON COUNTY
HAM-71-(156) (2.51)



PROPOSED STRUCTURE

TYPE: 3 Span Continuous Plate Girder with reinforced concrete deck and substructure

SPANS: 101.25', 105.83', 101.25', 1/4 brg. measured along base line.

ROADWAY: 36'-00" 1/2 curbs

SIDEWALK: 6'-0" Both sides

LOAD FREQUENCY: CF=400(57)

SKEW: None

WEARING SURFACE: 1" Monolithic concrete

APPROACH SLABS: AS-1-54 (25' long)

ALIGNMENT: Tangent

SUPERELEVATION: As shown

1986 Traffic Count ADT = 5400
E. Bound DHV = 460
W. Bound DHV = 380

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

SITE PLAN
BRIDGE No. HAM-71-0271
McGREGOR AVE. OVER I-71

H & E BRIDGE NO. 20

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
AL.T.	AL.T.	H.A.B.	J.H.O.	5/20/65	

GENERAL NOTES

- Symbol denotes Test Boring
- Abutments & Bearing & of Piers are parallel
- For Test Boring Data, see sheet 13 of 13
- For Bench Marks, see sheet 40

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FEB 20 1965

ESTIMATED QUANTITIES

ITEM	TOTAL	UNIT	DESCRIPTION	SUPER-STRUCTURE	WEST ABUTMENT	EAST ABUTMENT	PIER 1	PIER 2	GENERAL
E-2	Lump	Sum	Cofferdams, Cribbs, & Sheeting						Lump
E-2	1200	Cu. Yds.	Unclassified Excavation		400	380	130	290	
S-1	570	Cu. Yds.	Class "C" Concrete, Superstructure	570					
S-1	187	Cu. Yds.	Class "C" Concrete, Piers above Footings				88	99	
S-1	185	Cu. Yds.	Class "E" Concrete, Abutments above Footings		98	87			
S-1	307	Cu. Yds.	Class "E" Concrete, Footings		68.7	52.3	87.8	98	
S-4	222,991	Lbs.	Reinforcing Steel						
* S-7	560,000	Lbs.	Structural Steel	560,000					
* S-8	560,000	Lbs.	Field Painting of Structural Steel	560,000					
S-14	707	Lin. Ft.	Railing Type "C" (Aluminum Rail & Supports, Concrete Parapet)	619.2	54.8	33.0			
S-16	Lump	Sum	First Test Pile						Lump
S-18	4320	Lin. Ft.	Steel Piles, 10BP42		830	1150	540	1800	
S-25			Electric Lighting System (See Sh. No. 204)						
S-29	96	Cu. Yds.	Porous Backfill		63	33			
S-29	130	Lin. Ft.	6" Helical Perforated Corrugated Metal Pipe [Sec. M-6.4(h)] (Including Specials)		86	44			
S-29	107	Lin. Ft.	6" Helical Non-Perforated Corrugated Metal Pipe [Sec. M-6.4(h)]		47	60			
S-29	Lump	Sum	Drain Inlets, Including supports and Horizontal Collector System	Lump					
S-29	113	Lin. Ft.	8" Standard Pipe Downspout, Wrought Iron or Hot-dip Galvanized Steel, Including Specials.		14		44	55	
I-10	157	Sq. Yds.	Concrete Slope Protection		157				
I-10	325	Sq. Yds.	Crushed Aggregate Slope Protection			325			
S-101	570	Each	Water-Reducing, Set-Retarding Admixture	570					

* ITEMS CHARGEABLE TO CINCINNATI GAS & ELECTRIC CO			
ITEM	UNIT	DESCRIPTION	TOTAL
S-7	Lbs.	Structural Steel	4940
S-8	Lbs.	Field Painting of Structural Steel	4940

GENERAL NOTES

REFERENCE shall be made to Standard Drawing I-15, No. 6

POROUS BACKFILL: 1'-6" thick, full length of abutments and wings of West Abutment shall extend up to the underside of the approach slab or to the finished ground surface.

MACHINE FINISH: The Concrete bridge deck shall be finished by the use of a finishing machine.

RAILROAD AERIAL LINES will be relocated by the railroad. The contractor shall use all precautions necessary to see that the lines are not disturbed during the construction stage and shall cooperate with the railroad in the relocation of these lines. The cost of the relocation shall be included in the railroad force account work.

CONSTRUCTION CLEARANCE of 21'-0" vertically above the top of the railroad rails and 8'-0" horizontally from the center of tracks shall be maintained at all times.

SHEETING AND BRACING: Before construction is started, eight sets of prints showing details of the sheeting and bracing to be used for excavation adjacent to the railroad tracks shall be submitted to the Director for approval by the Department of Highways and by the Railroad Company.

ALIGNING RAILROAD TRACKS: After the Contractor has completed all excavation and backfill adjacent to the railroad tracks in compliance with Sec. E-2.04 and E-2.08 of the Construction and Material Specifications, subject to the Supervision of the Railroad Company, nothing in Sec. E-2.04, E-2.08 or G-8.07 of the Specifications shall be construed to hold the Contractor liable for aligning and resurfacing the railroad tracks.

Piles shall be driven with a hammer of not less than 11,000 ft. lbs. per blow to firm contact with limestone. If the length of penetration is approximately equal to the depth of limestone according to the bridge foundation investigation report, the firm contact shall be considered as attained when the capacity according to the formula in Sec. S-18.05 is not less than the following value for a pile hammer of the indicated energy rating.

- 50 Tons per pile for West Abutment using an 11,000 ft. lb. hammer
- 45 Tons per pile for West Abutment using a 15,000 ft. lb. hammer
- 50 Tons per pile for Piers No. 1 & 2 using an 11,000 ft. lb. hammer
- 45 Tons per pile for Piers No. 1 & 2 using a 15,000 ft. lb. hammer
- 50 Tons per pile for East Abutment using an 11,000 ft. lb. hammer
- 45 Tons per pile for East Abutment using a 15,000 ft. lb. hammer

If the energy rating of the hammer is between the rating as shown above, the required formula capacity shall be determined by interpolation. The design load is 35 Tons per pile.

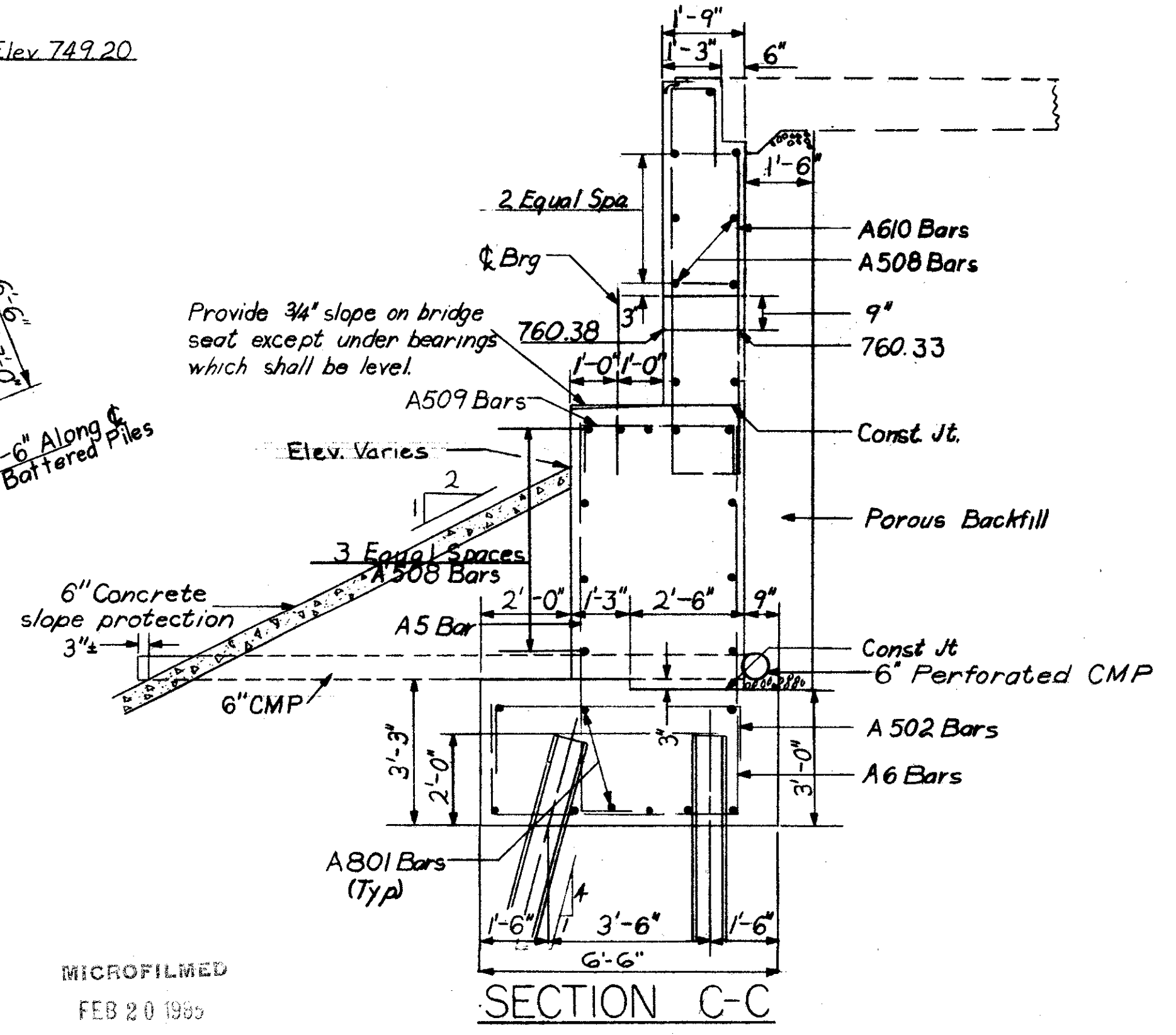
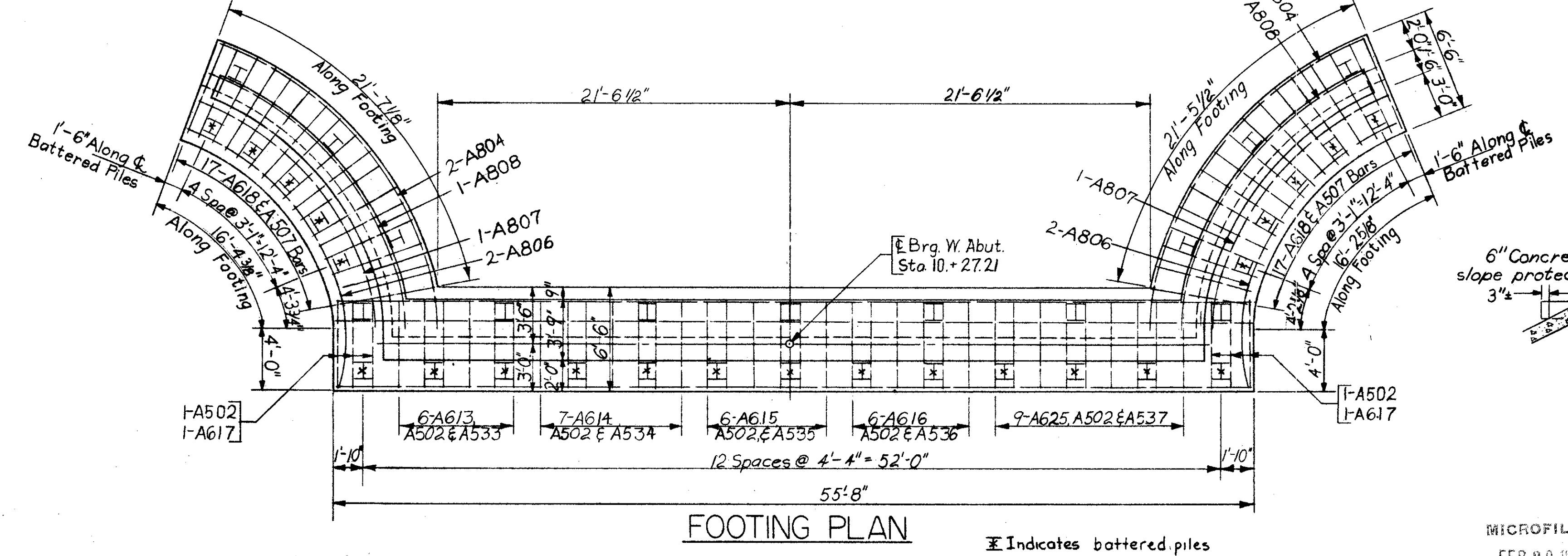
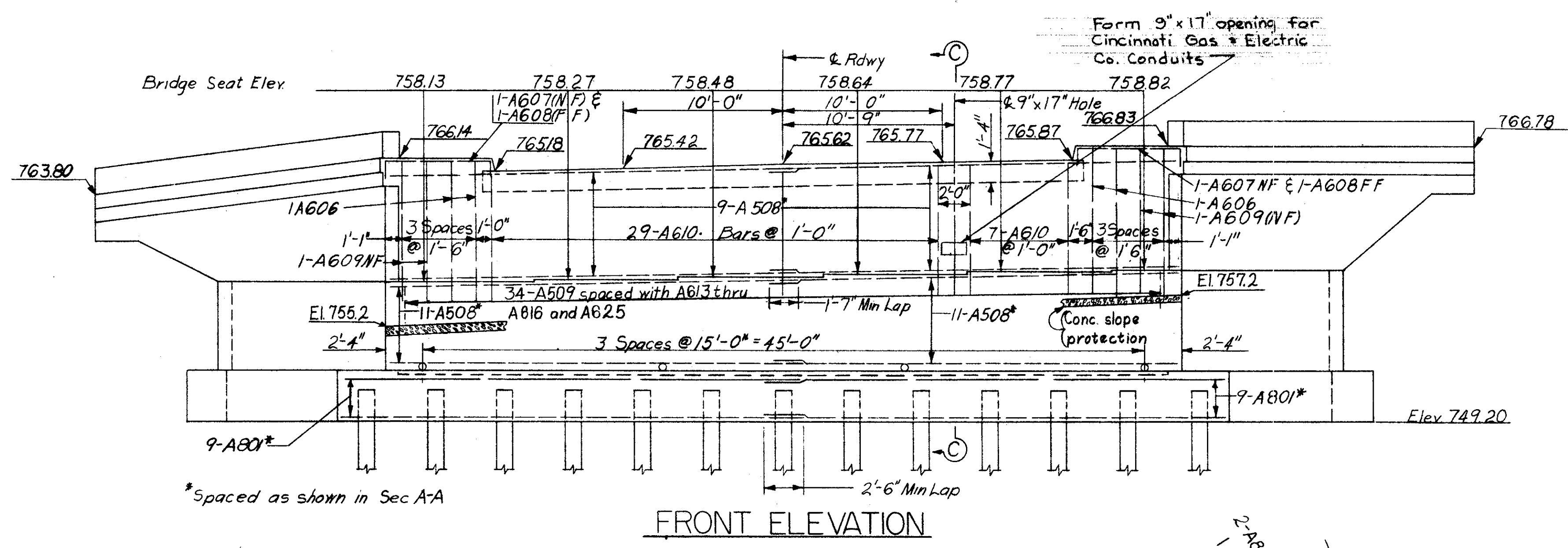
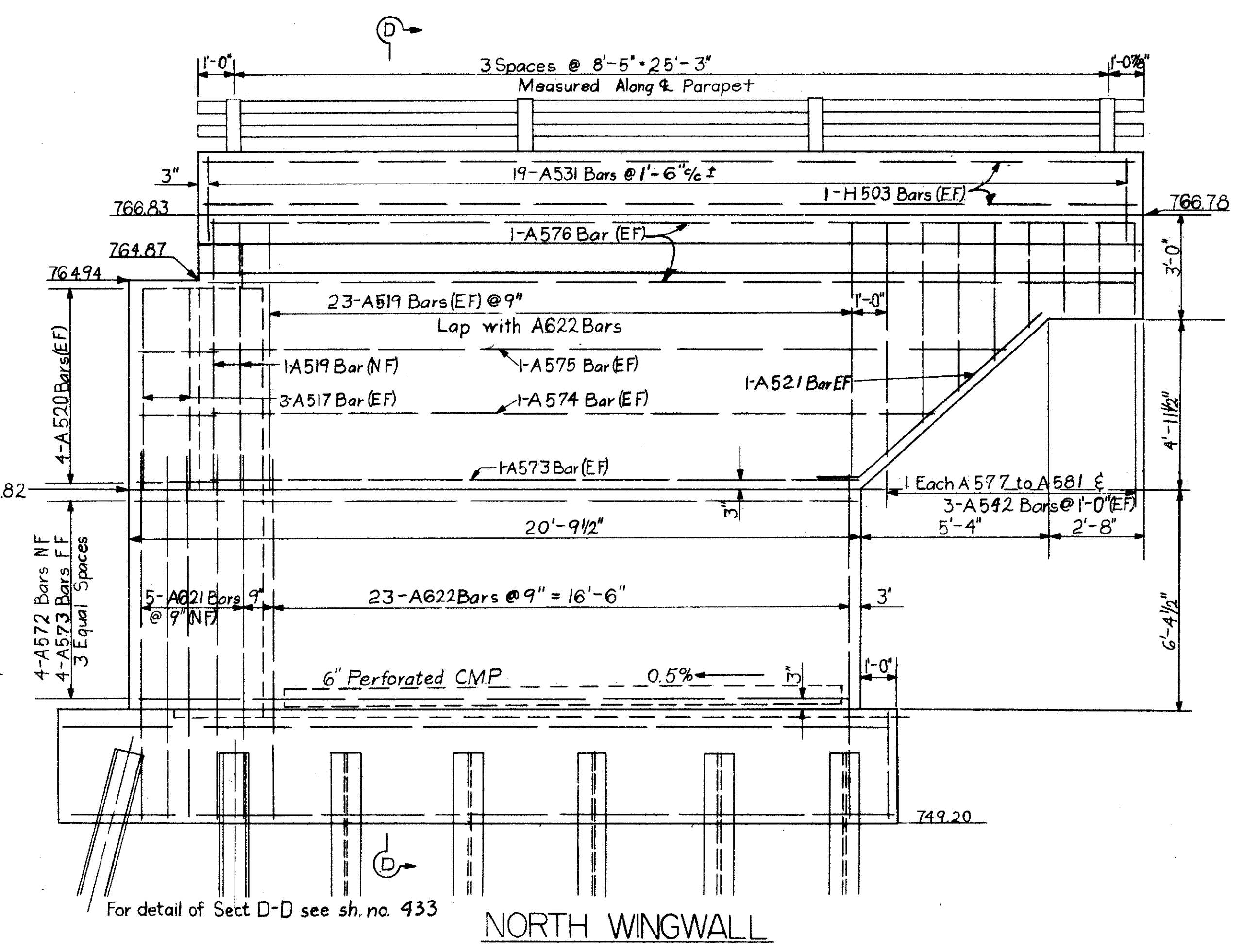
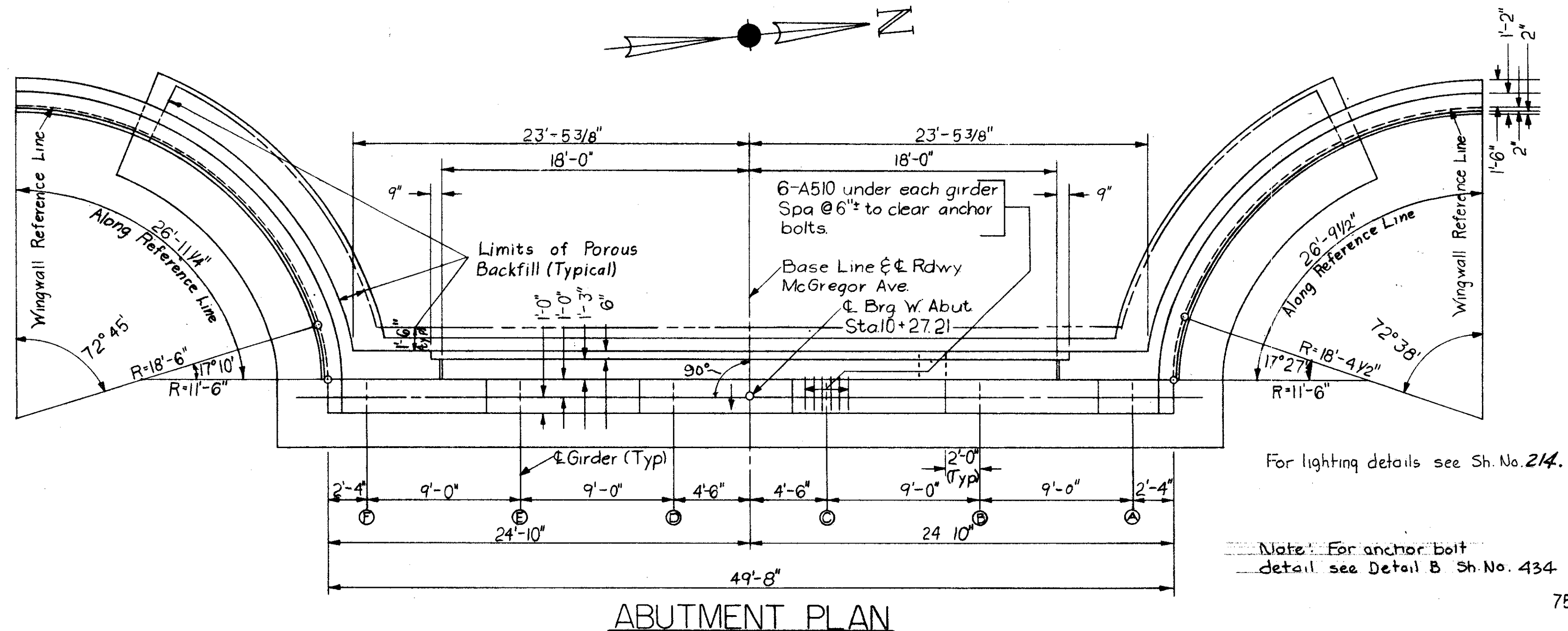
ADDITIONAL NOTES: For additional notes see notes 1, 2, 3, 5, 6, 7, & 8, Sh. No. 443

Design Loading - CF 2000 (57)
 Concrete Class C - basic unit stress 1,333 psi.
 Concrete Class E - basic unit stress 1,133 psi.
 Structural Steel - ASTM A 36 - basic unit stress 20,000 psi.
 (Except for Piles, ASTM A 7 and A 373 steel not permitted)
 Reinforcing Steel - ASTM A 15, A 16, A 160, Deformed, Intermediate or Hard Grade. Basic unit stress 20,000 p.s.i.

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FEB 20 1965

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
ESTIMATED QUANTITIES AND GENERAL NOTES BRIDGE No. HAM-71-0271					
H&E BRIDGE No 20					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	CIB		JHO	11.1.3. 5-20-65	

HAMILTON COUNTY
HAM-71(156) (2.51)



Notes: Bars A538 thru A541, A572 thru A576 and H502, H503 will be bent in the field. For general notes see sh No 434

Work this sheet with sheet 433

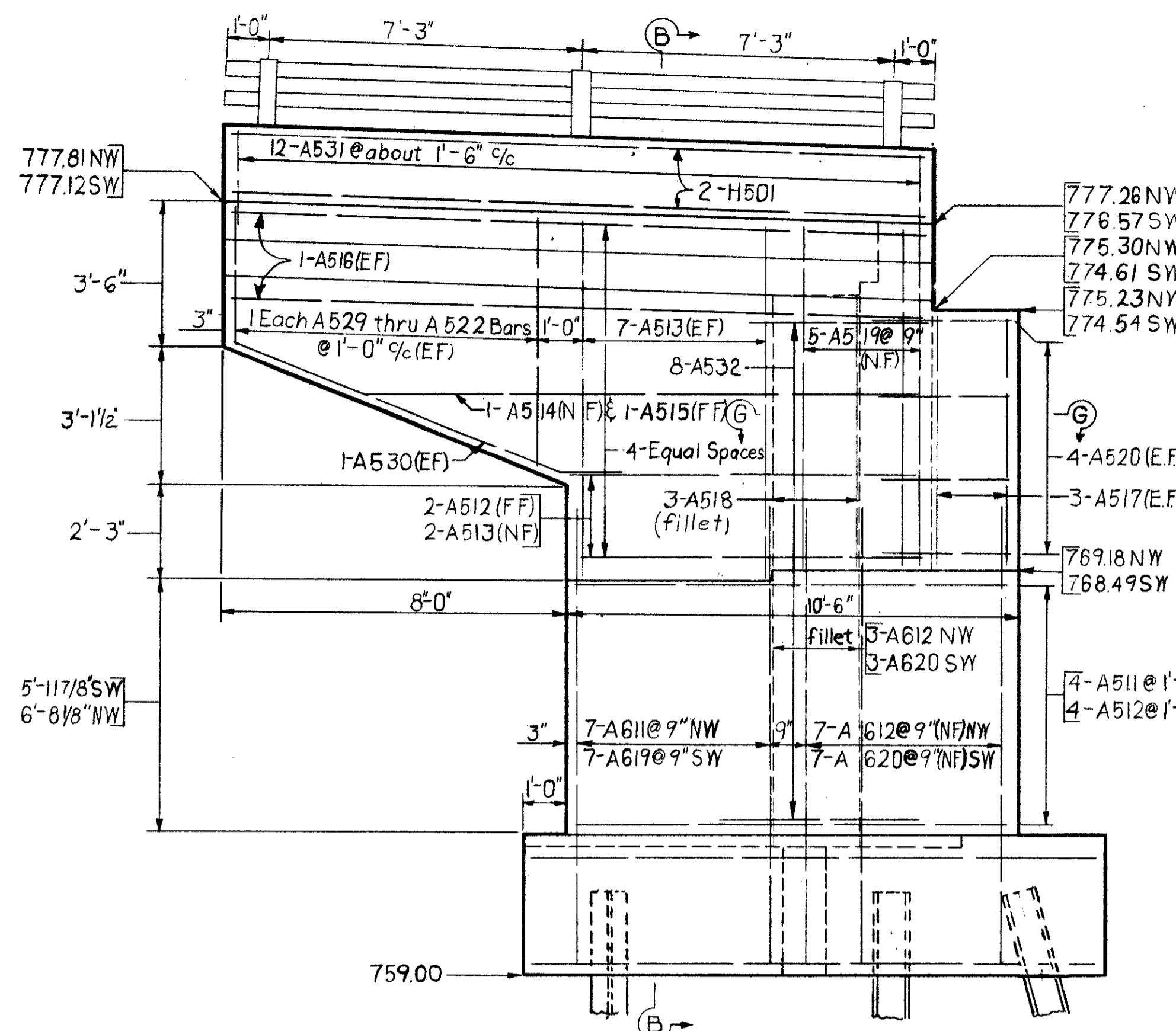
HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

WEST ABUTMENT
BRIDGE No HAM-71-0271

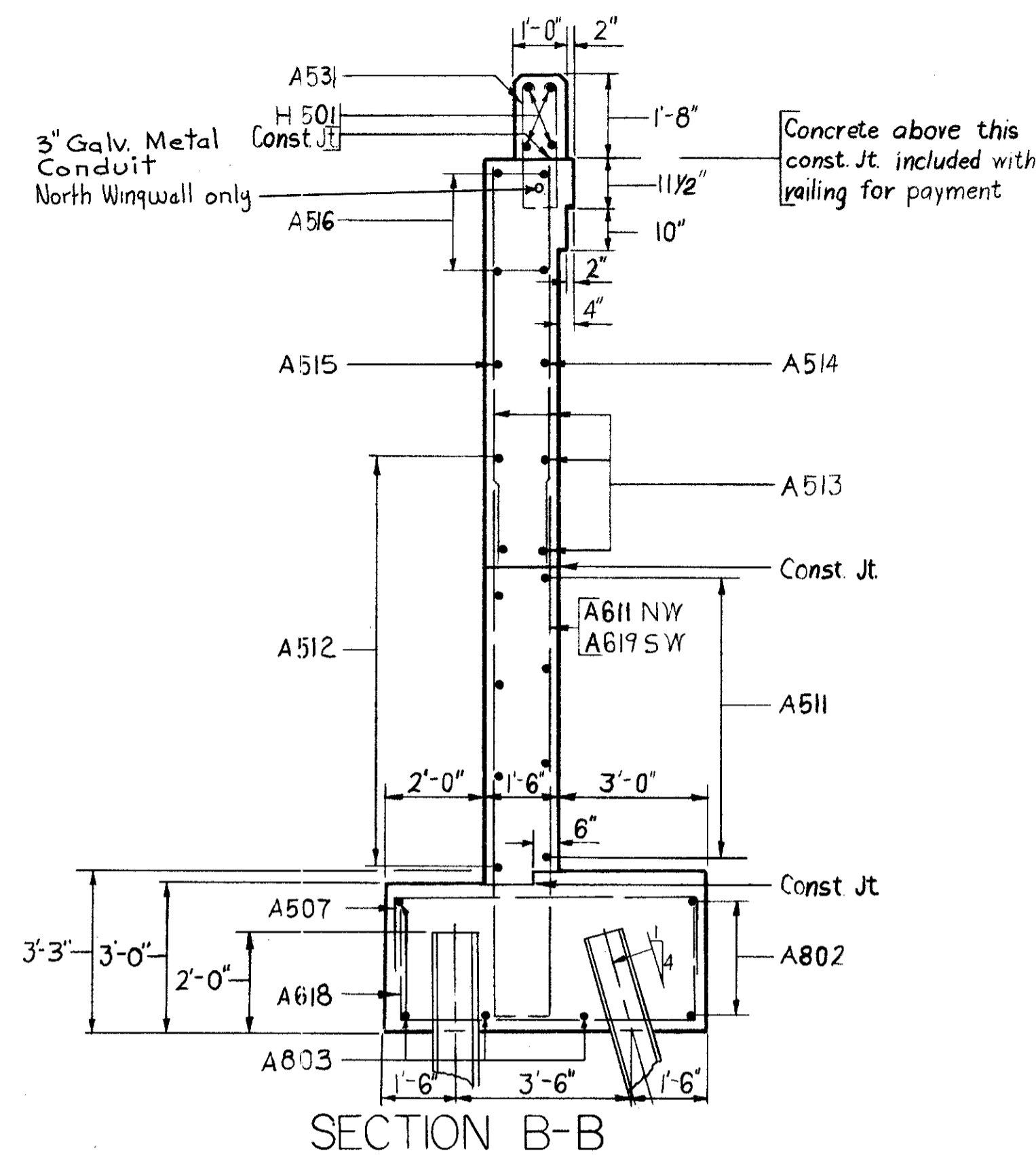
H&E BRIDGE No. 20

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
	ME		JHO	H. A. S. 5-20-65	

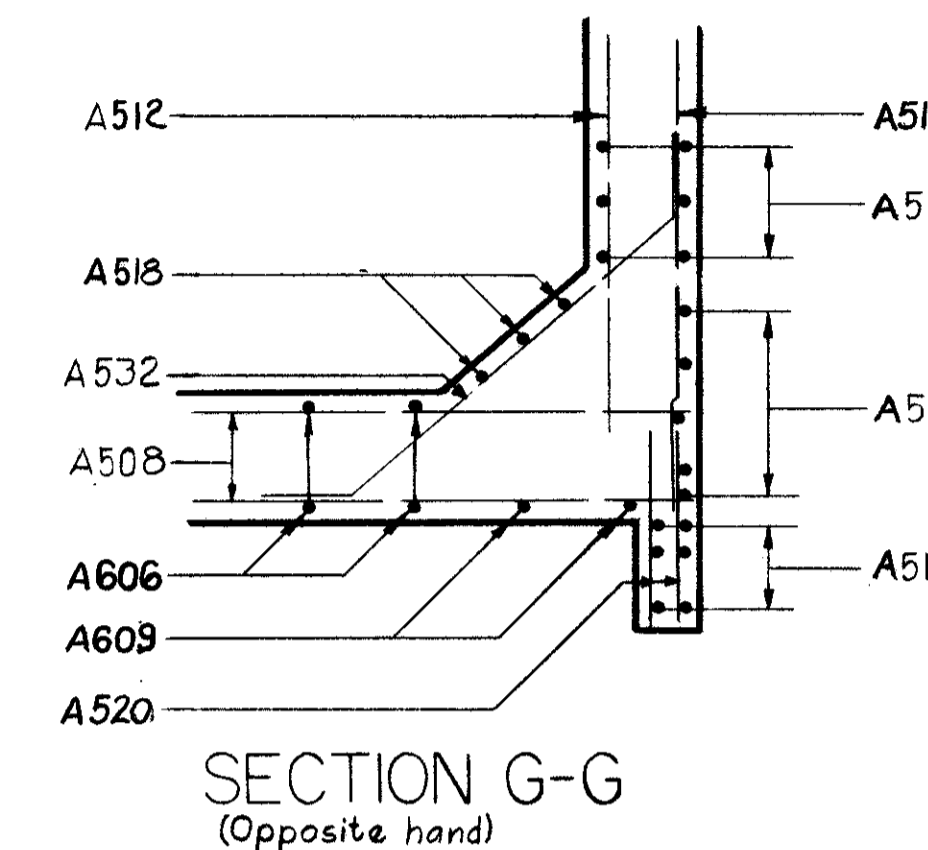
MICROFILMED
FEB 20 1965



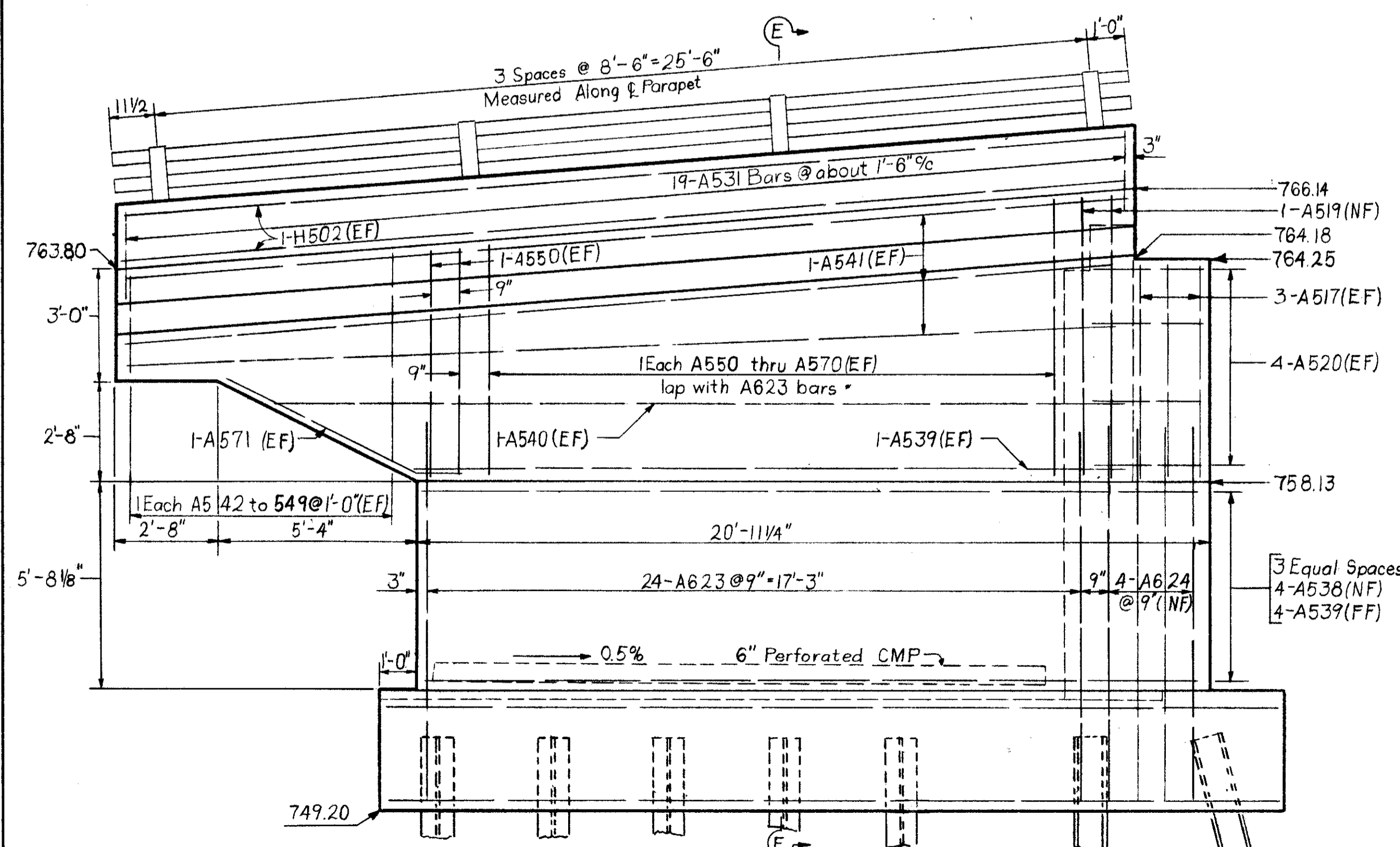
EAST ABUTMENT WINGWALL ELEVATIONS



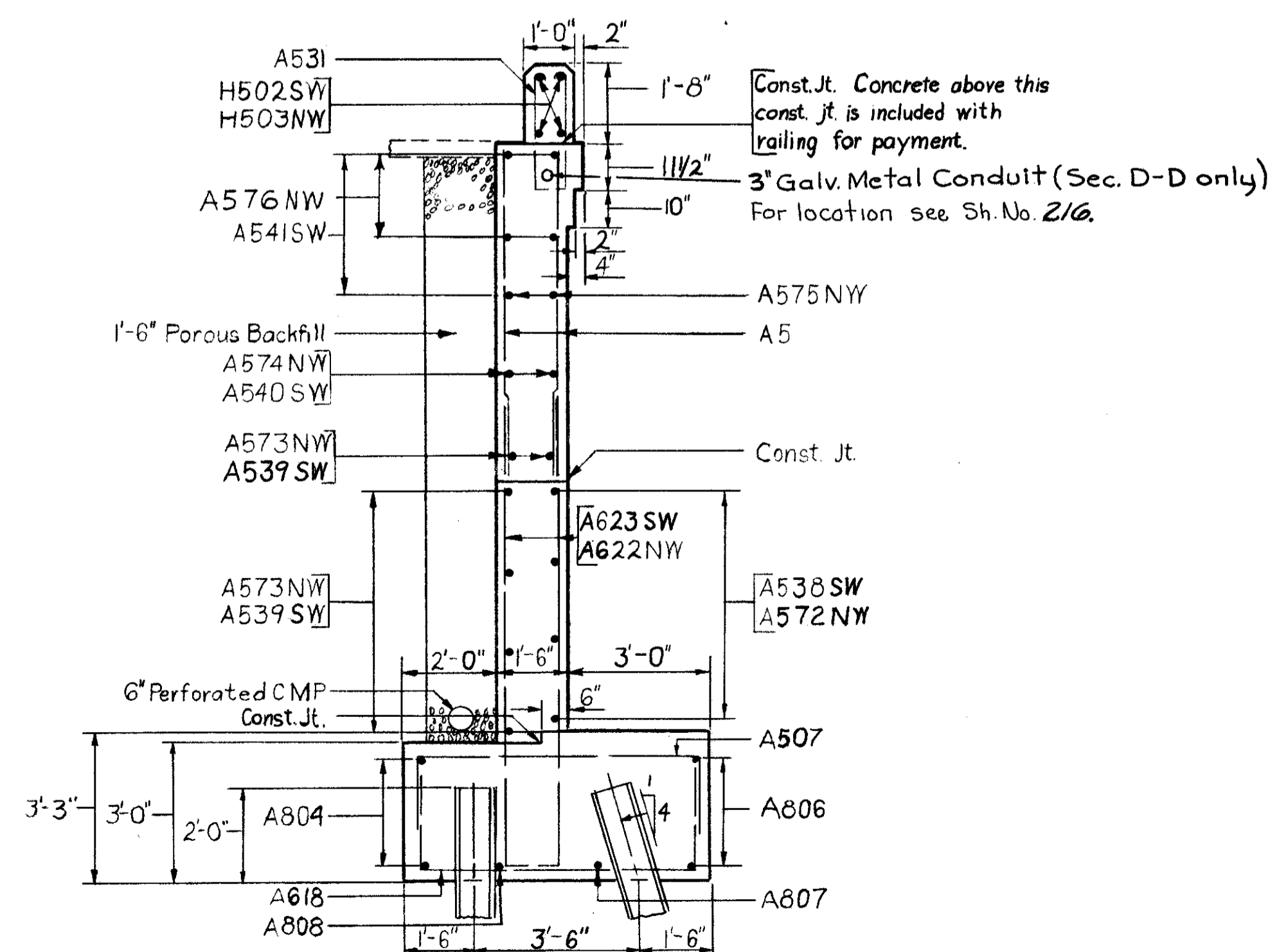
SECTION B-B



SECTION G-G
(Opposite hand)



WEST ABUTMENT SOUTH WINGWALL ELEVATION



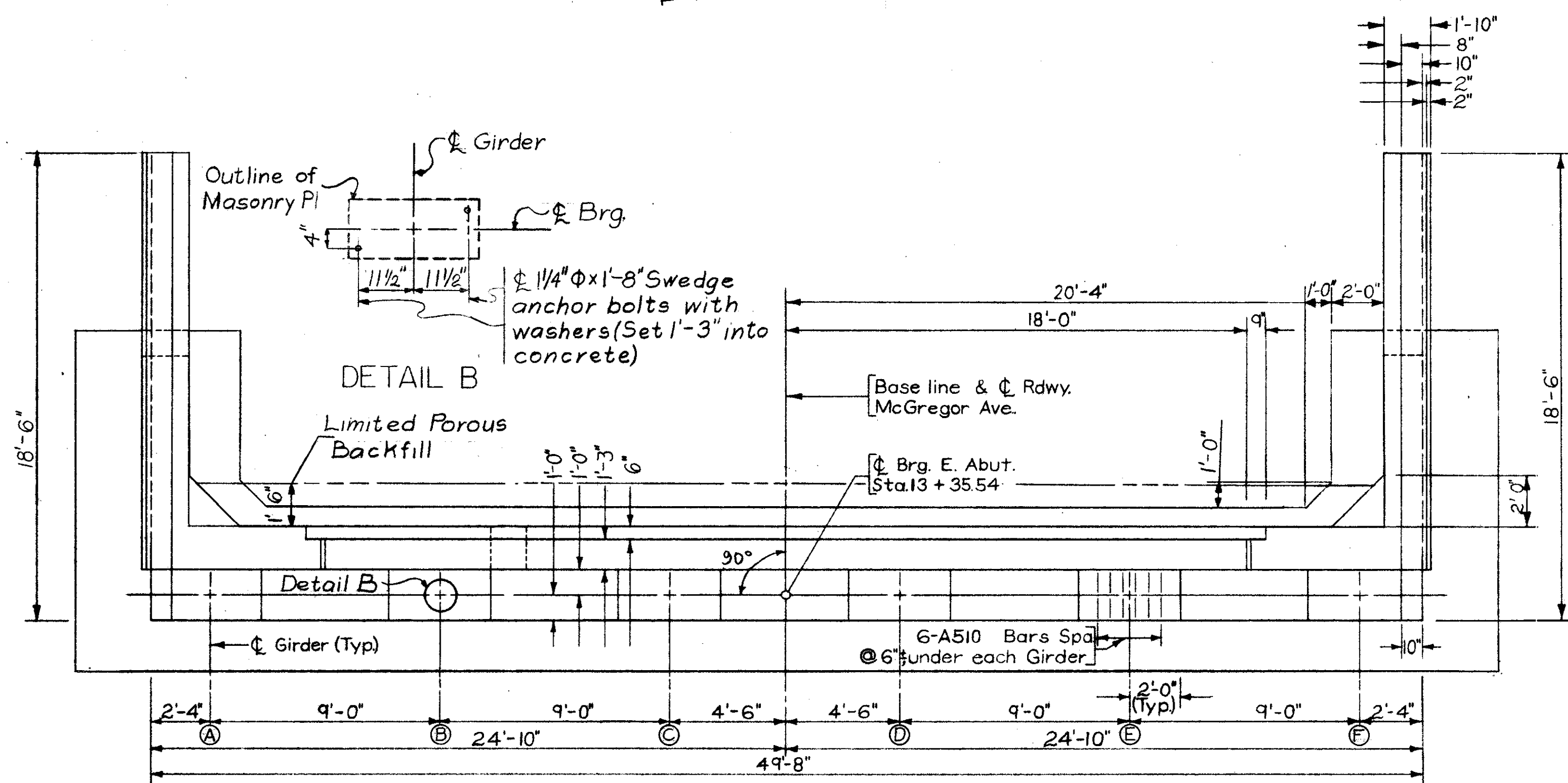
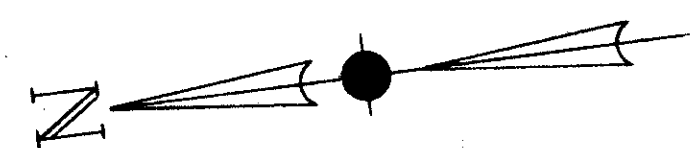
SECTION E-E SOUTH WINGWALL
SECTION D-D NORTH WINGWALL
For location of Sect D-D see sh. no. 432.

Work this sheet with Sheets 432 & 434

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
ABUTMENT WINGWALL DETAILS				
BRIDGE No. HAM-71-0271				
H&E BRIDGE NO. 20				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	ME		JHO	H.A.S. 5-20-65
				REVISED

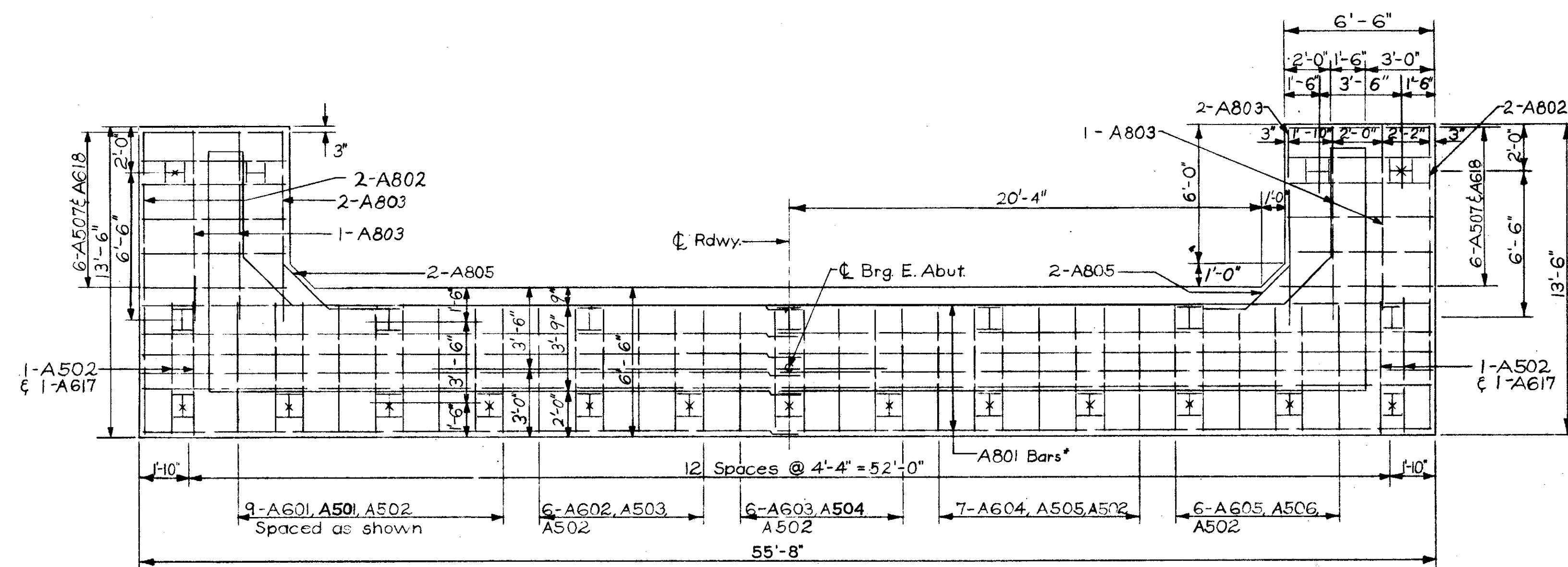
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FEB 20 1985

HAMILTON COUNTY
HAM-71-(1.56)(2.51)

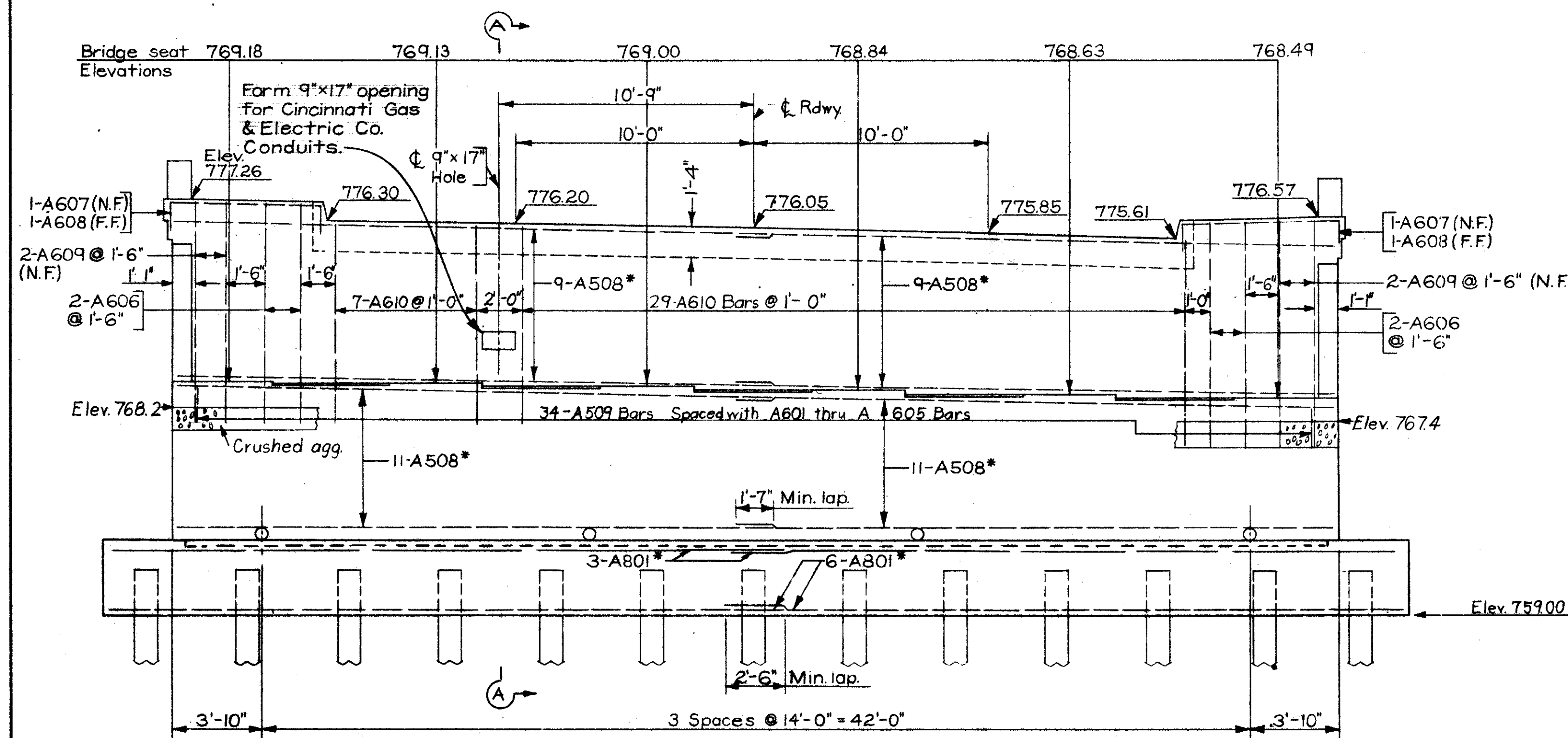


For lighting details see Sh. No. 214.

ABUTMENT PLAN

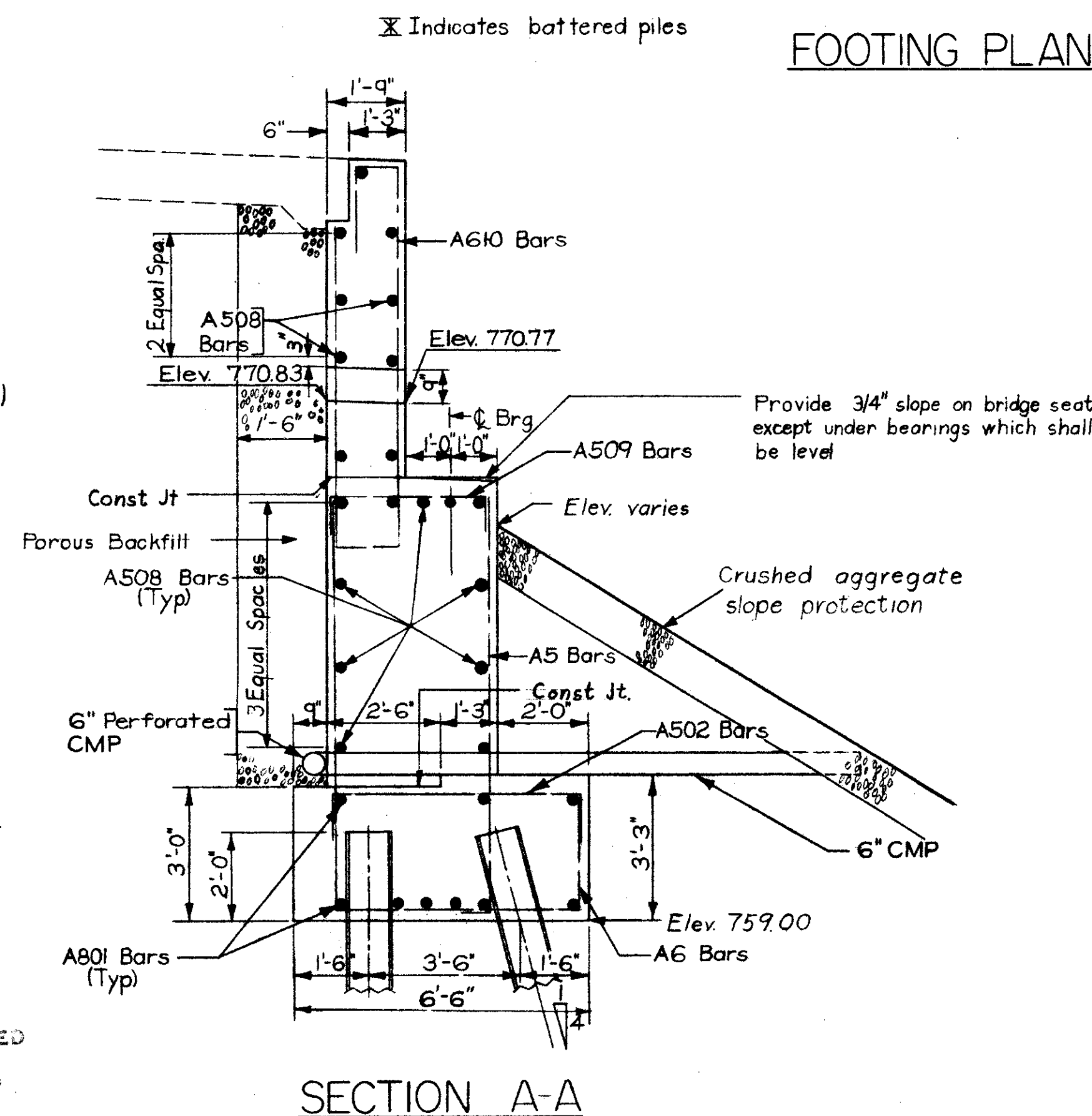


FOOTING PLAN



* Spaced as shown in Sec. A-A

FRONT ELEVATION



SECTION A-A

Notes:
N.F. denotes near face. F.F. denotes far face. E.F. denotes each face.
Provide 3" clearance to reinforcing steel in footing, and 2" clearance to reinforcing steel in wall, minimum.
Special care shall be taken in placing steel in the abutment seat, so that it will not interfere with the drilling of anchor bolt holes.
Top of backwall elevations shown are at the near face of backwall.
Concrete above bridge seat constr. Jt shall be placed after superstructure steel is in place.
For roadway & sidewalk end finish details see Sh. 437
For nailing details see Sh. 433
All piles are 10 BP42.

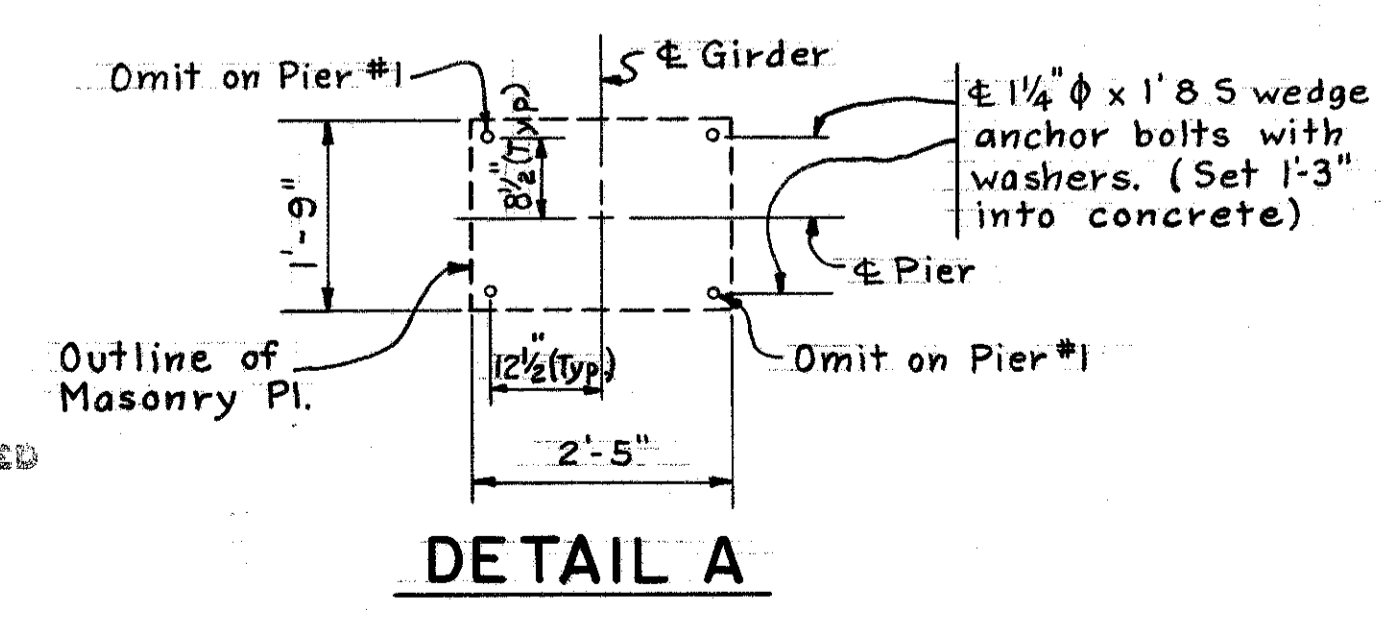
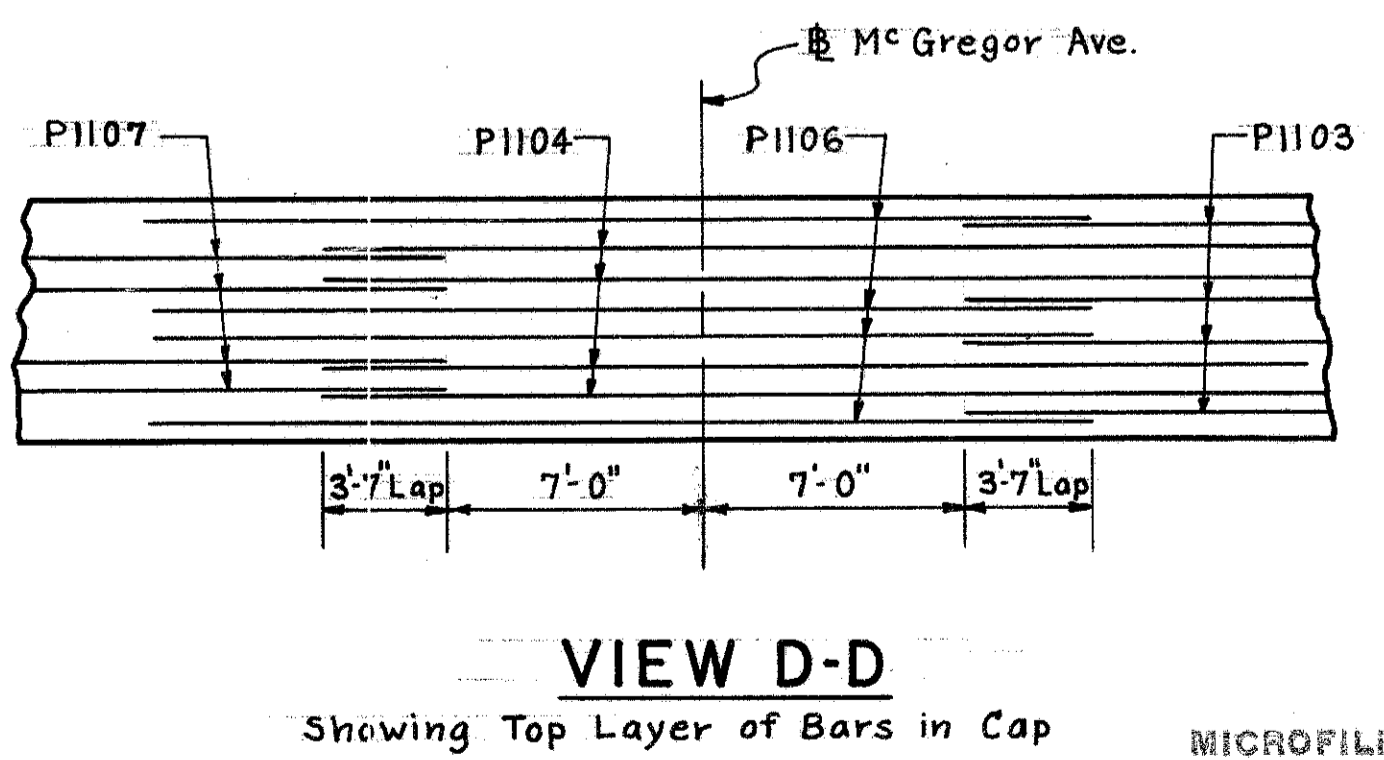
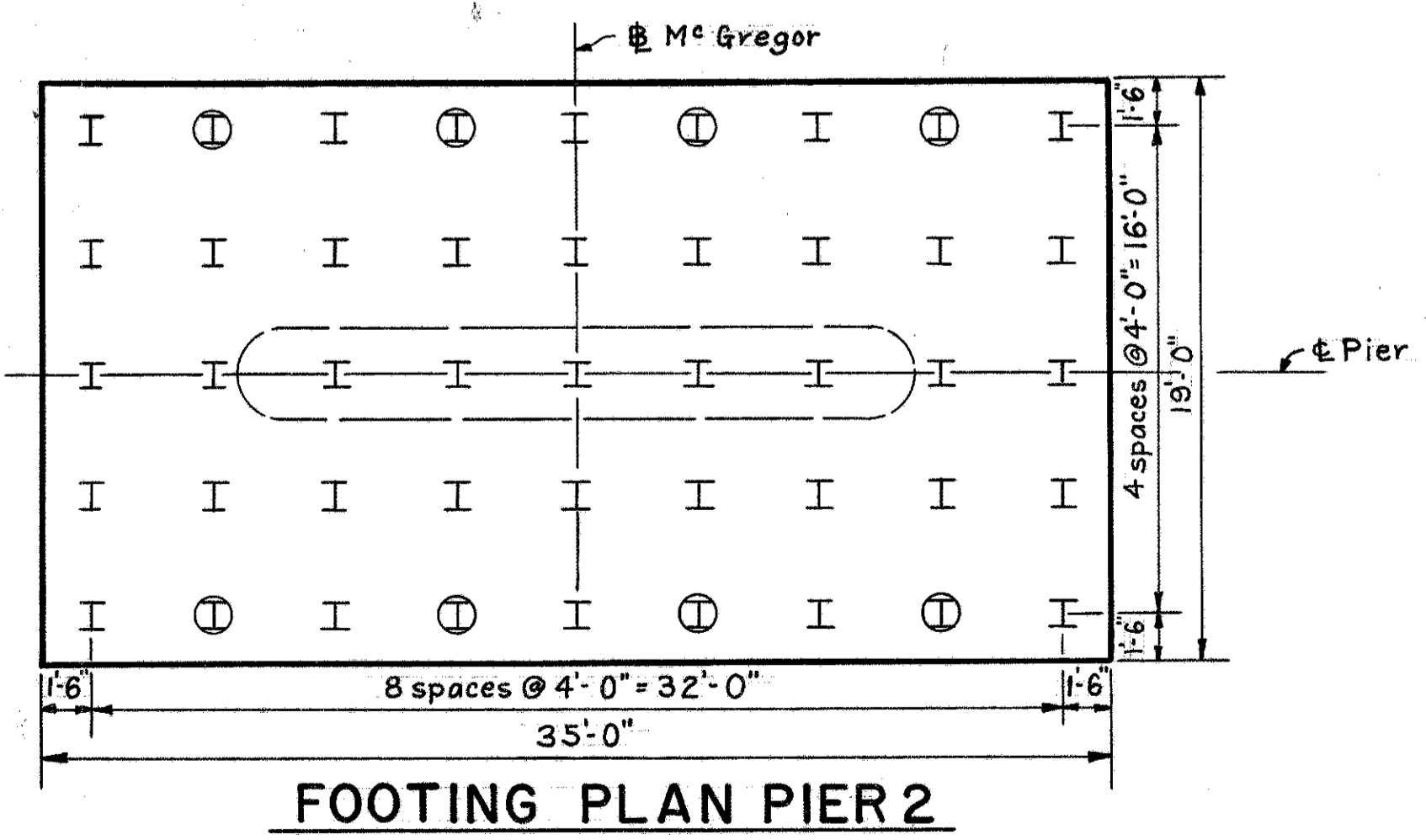
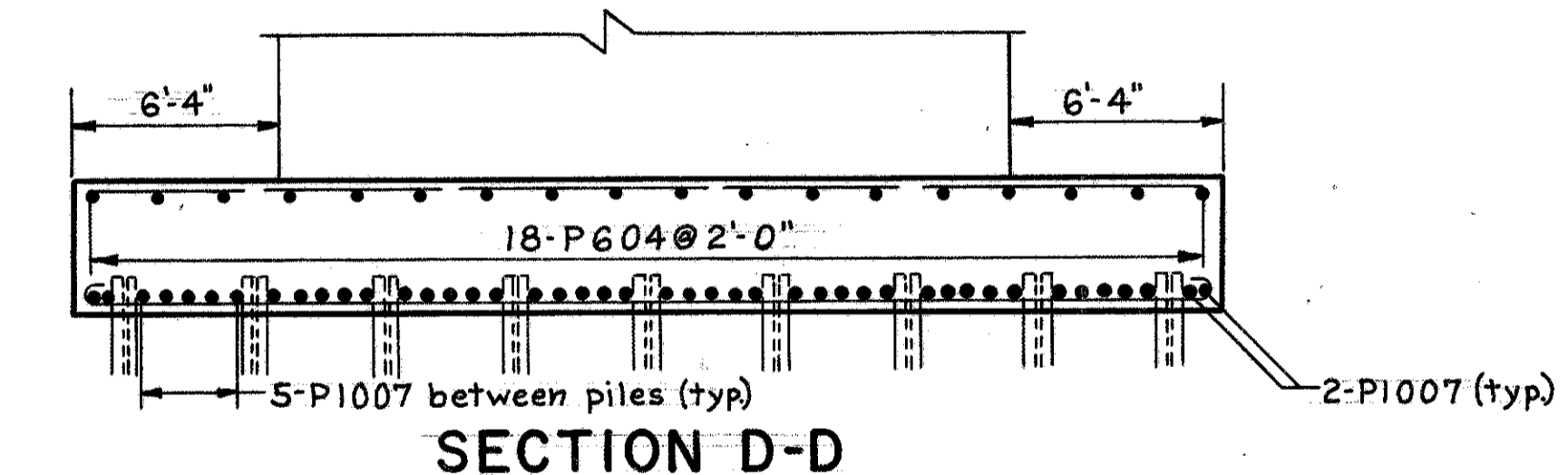
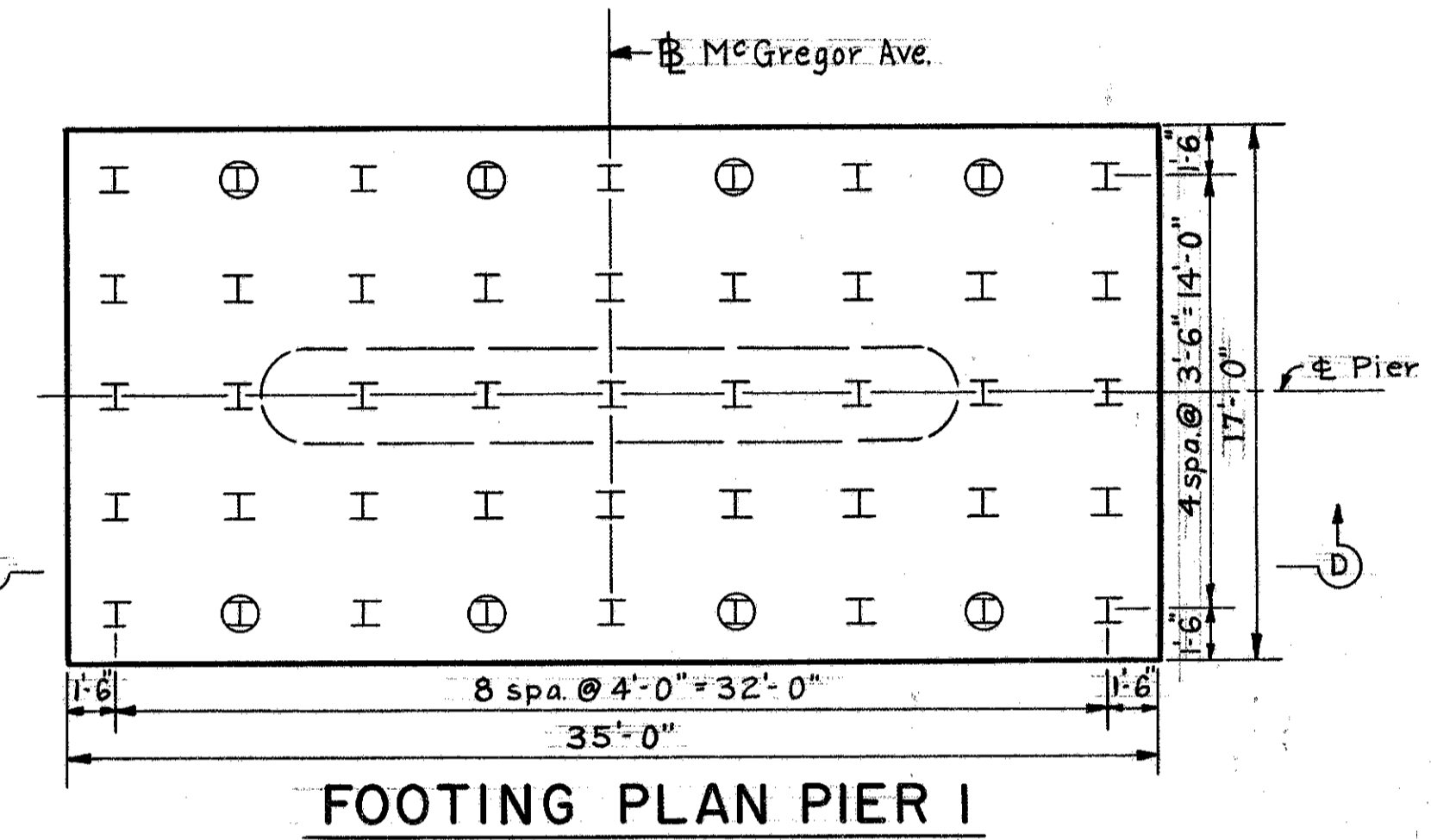
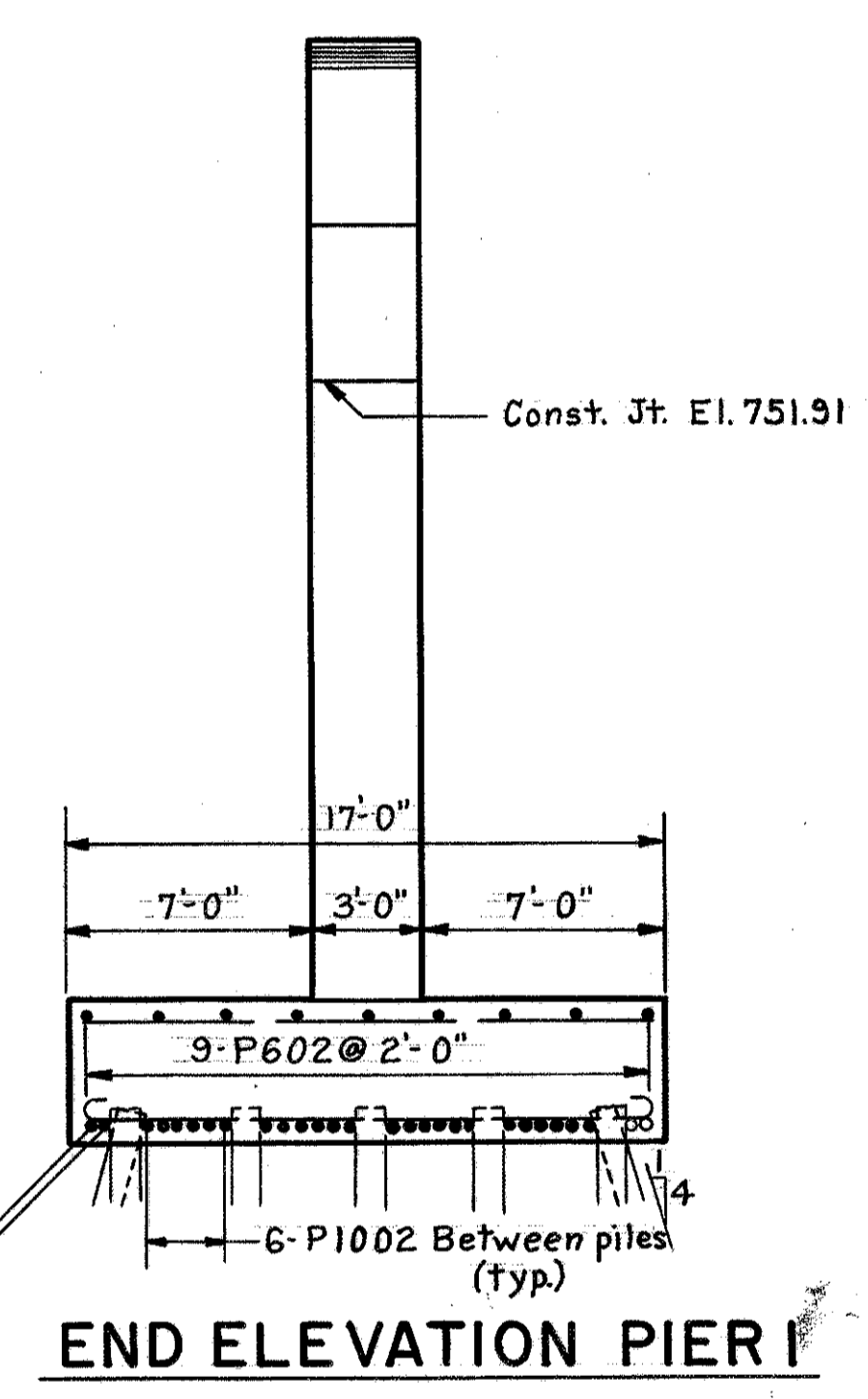
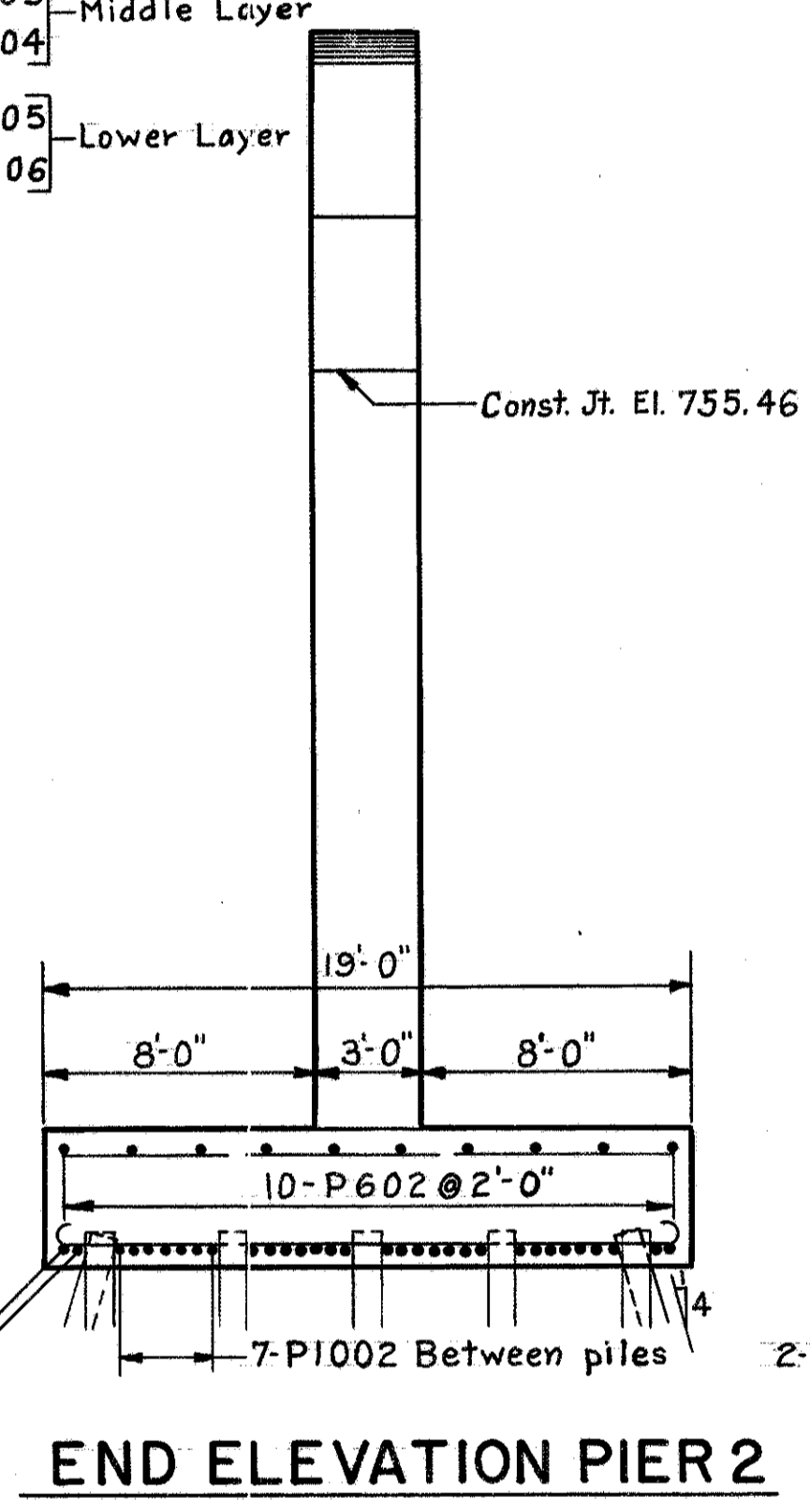
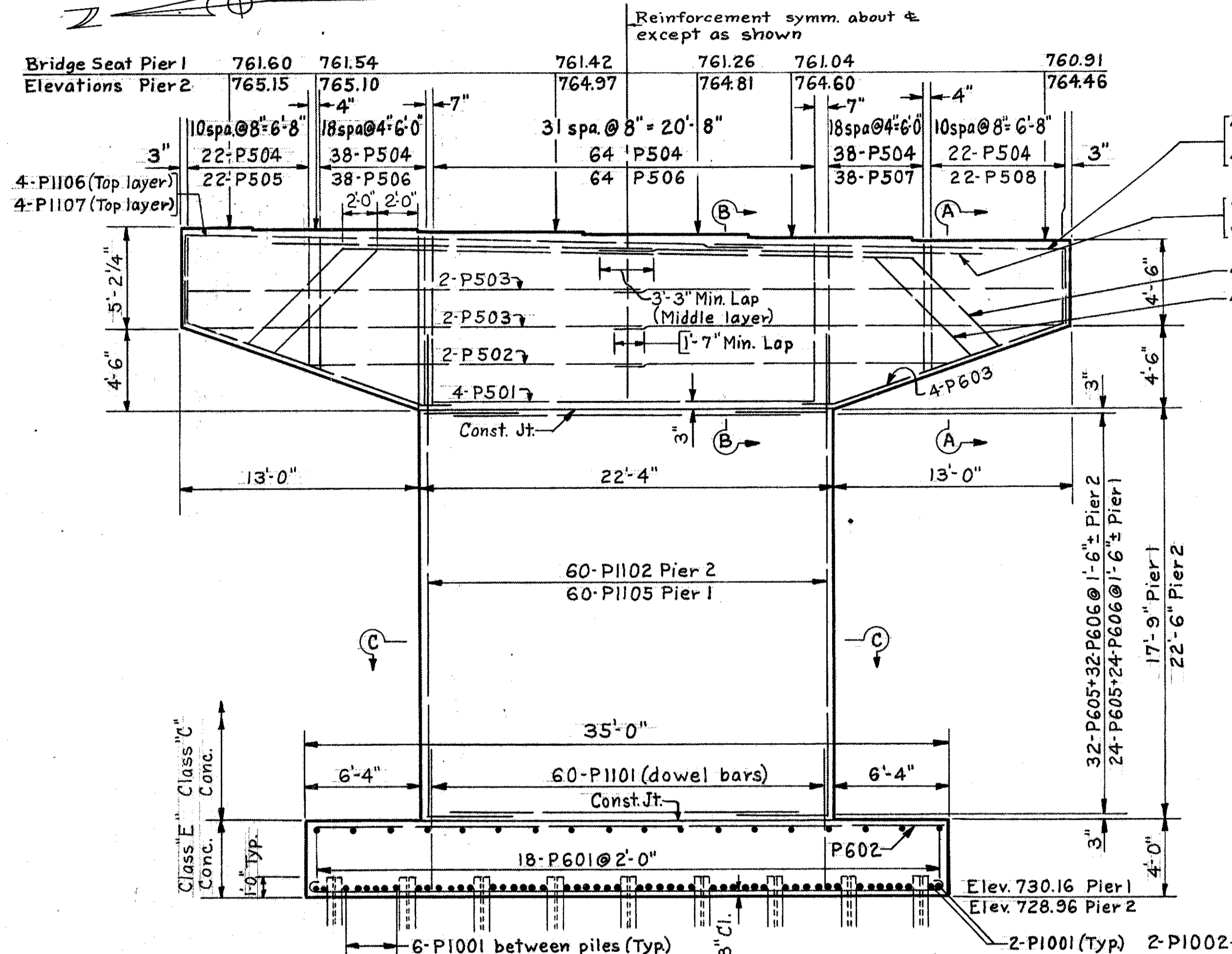
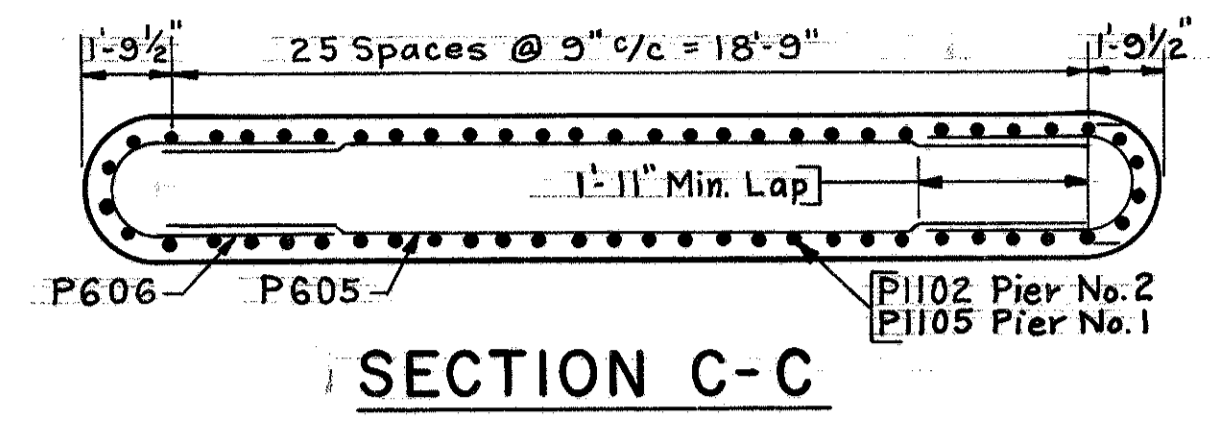
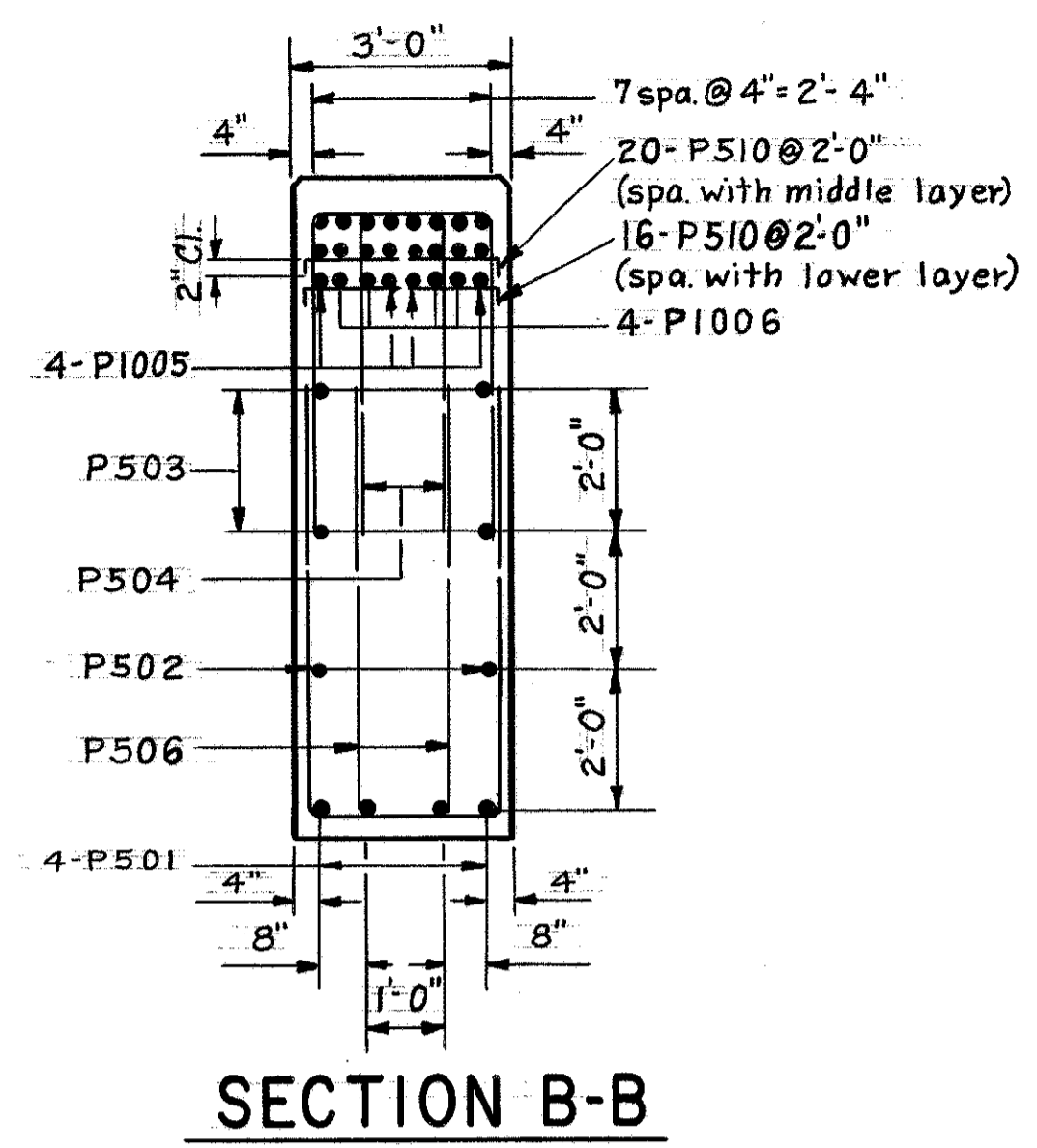
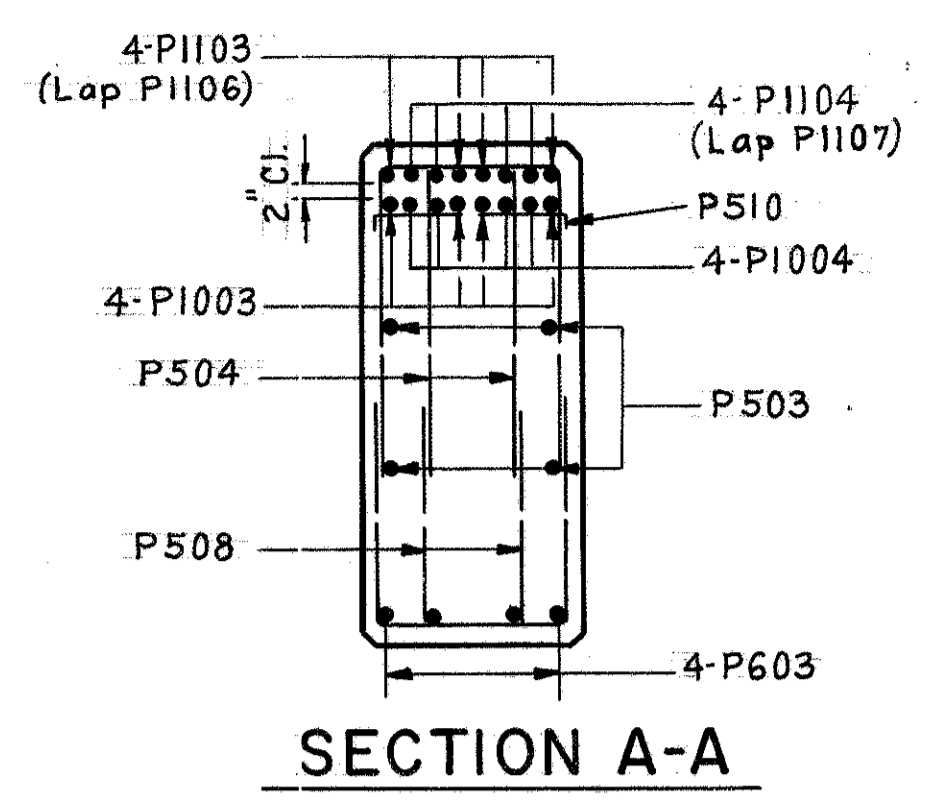
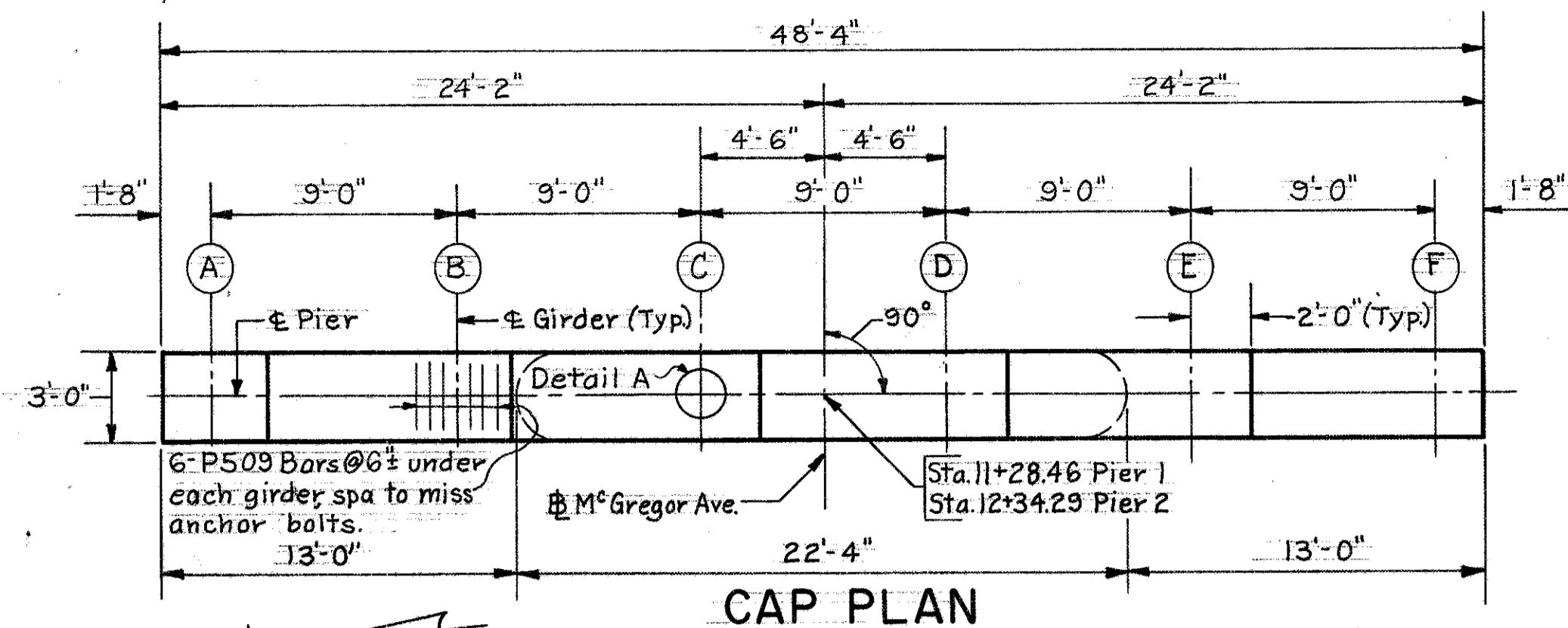
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FEB 20 1985

Work this sheet with Sh. No. 433

HAZLET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
EAST ABUTMENT BRIDGE No HAM-71-0271					
H&E BRIDGE No. 20					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISION
CIB			JHO	74. A. 8. 5-20-65	

MICROFILMED
FEB 20 1985

HAMILTON COUNTY
HAM-71 (1-56) (2.51)



Notes: ⊕ denotes battered piles. All piles to be 10 BP 42. All reinforcing steel shall be 2" clear from face of concrete except as shown. Special care shall be taken in placing reinforcing in cap so that it will not interfere with the masonry plate anchor bolts. Special care shall be taken in placing reinforcing steel in top of footing of Pier No. 1 so as to avoid interference with the drilling of anchor bolt holes for anchoring guard rail posts to footing as shown on Standard Drawing I-15 No. 6.

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

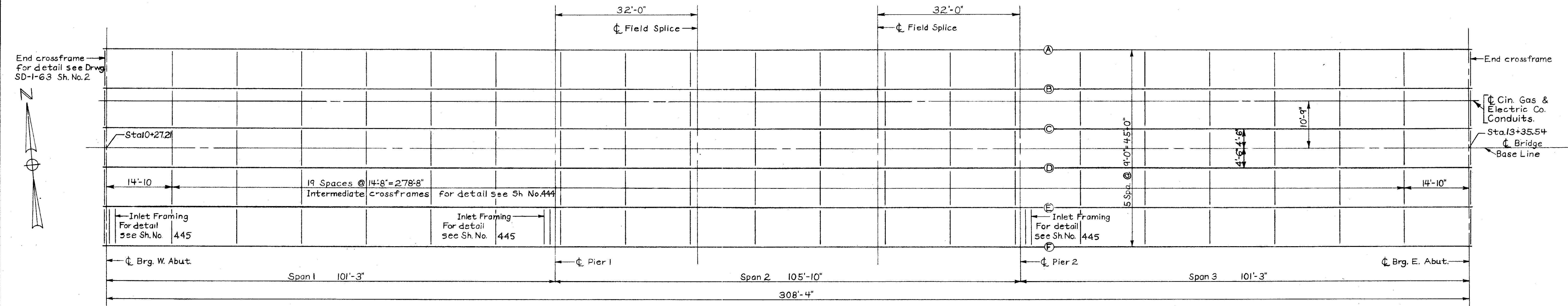
PIERS 1 & 2

BRIDGE NO. HAM-71-0271

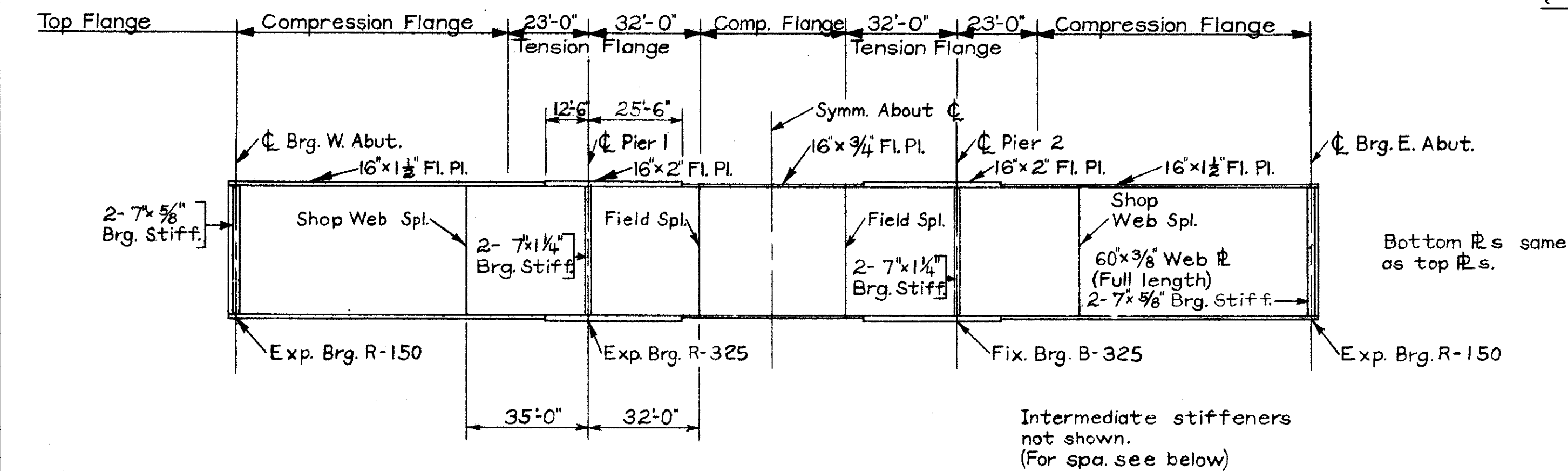
H & E BRIDGE No. 20

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	ME	RS	JHO	N.A.S. 5-20-65	

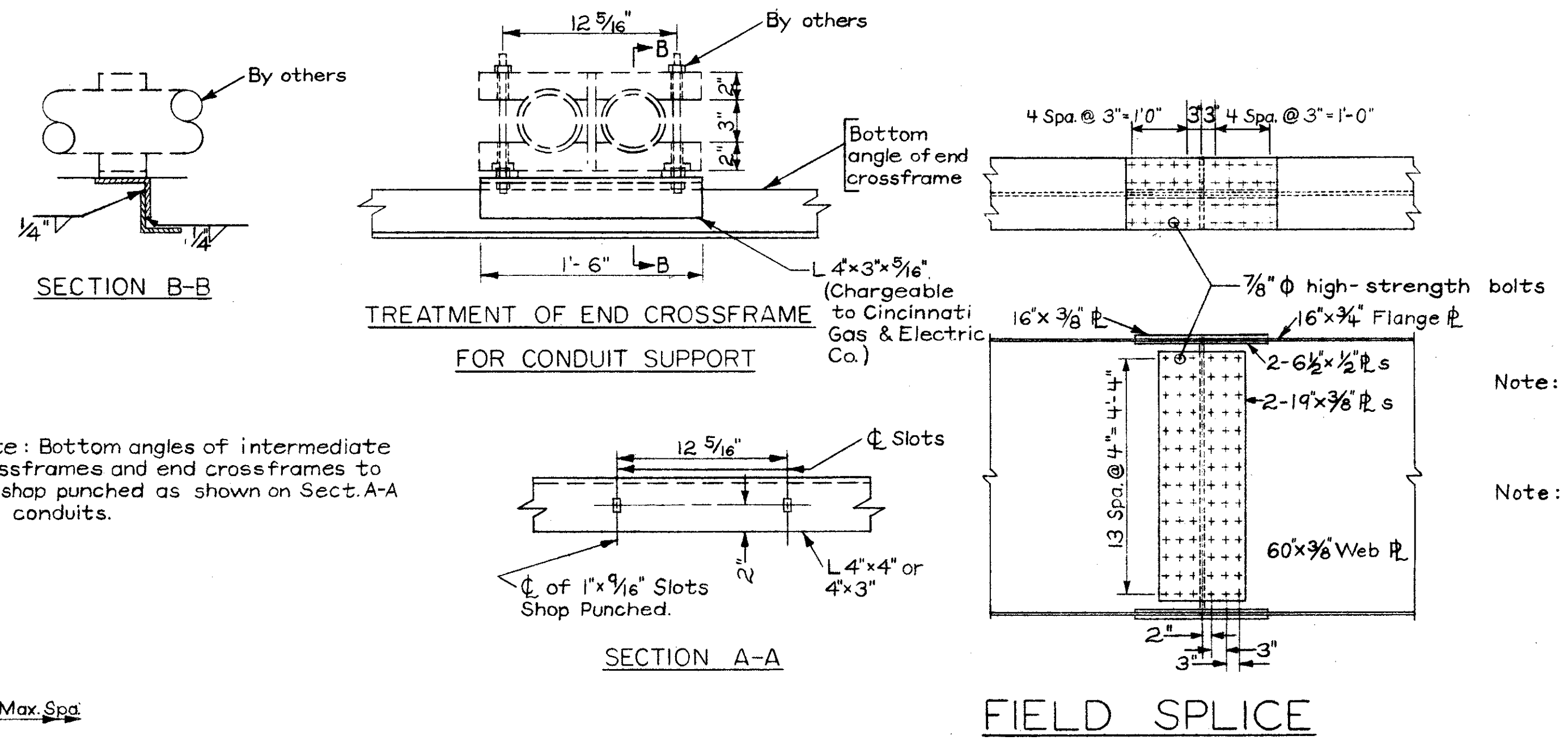
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FRAMING PLAN



GIRDER ELEVATION

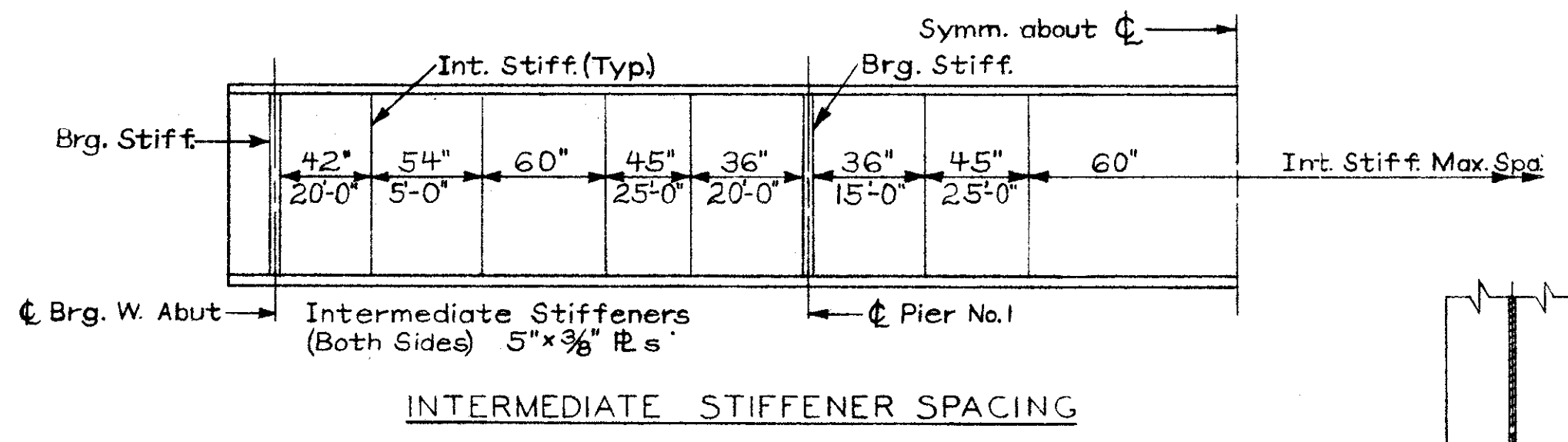


Note: Bottom angles of intermediate crossframes and end crossframes to be shop punched as shown on Sect. A-A for conduits.

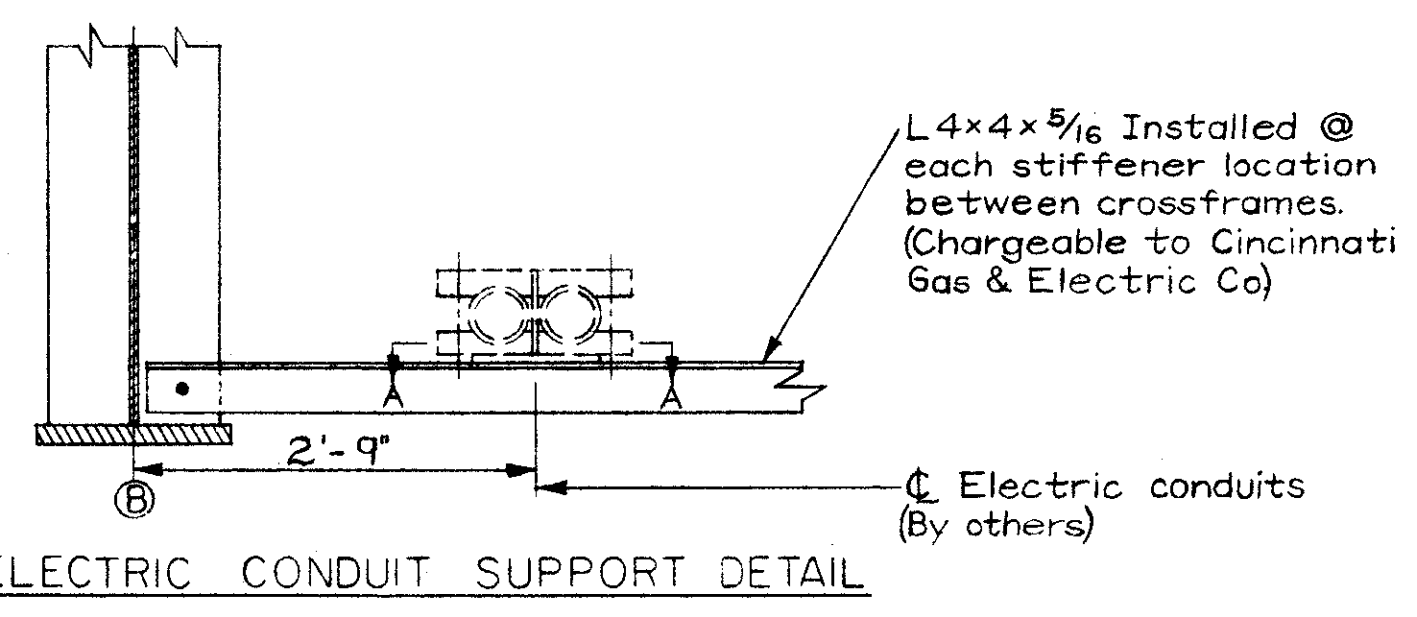
Note: Contact surface of splice shall be free of all oil or paint.

Note: Bottom flange splice material same as top flange splice material.

For bearing details see Sh. No. 438
For typical plate girder elevation see Sh. No. 444
For detail of roadway and sidewalk end dam details see Sh. No. 437
For treatment of ends of bridge see Drawing No. SD-1-63, Sh. No. 2, "Longitudinal Section."

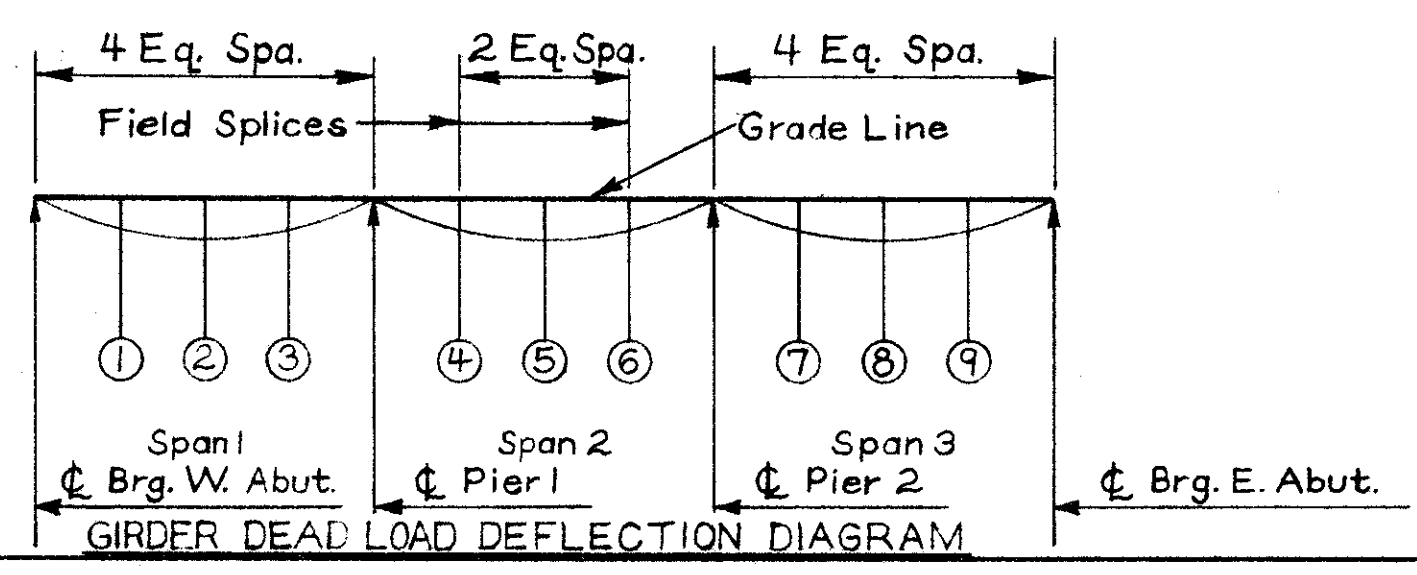


INTERMEDIATE STIFFENER SPACING



Note: Camber girders by cutting webs to a smooth curve. Camber shall be equal to the total dead load deflection ordinates shown in box.

Girder	Deflection Due to:	Location								
		①	②	③	④	⑤	⑥	⑦	⑧	⑨
A&F	Steel Wt.	3/16	3/16	1/8	0	1/16	0	1/8	3/16	3/16
	Remaining DL	1/16	3/16	3/8	0	3/16	0	3/8	1/16	1/16
	Total D.L.	7/8	1	1/2	0	1/4	0	1/2	1	7/8
Bthru E	Steel Wt.	1/8	3/16	1/8	0	1/16	0	1/8	3/16	1/8
	Remaining DL	5/8	1/16	3/8	0	3/16	0	3/8	1/16	5/8
	Total D.L.	3/4	7/8	1/2	0	1/4	0	1/2	7/8	3/4



ITEMS CHARGEABLE TO CINCINNATI GAS & ELECTRIC CO.			
Item	Unit	Description	Amount
S-7	Lbs.	Structural Steel	4940

MICROFILMED
FEB 20 1985

HAZLET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

STRUCTURAL STEEL DETAILS
BRIDGE No. HAM-71-0271

H&E BRIDGE No. 20

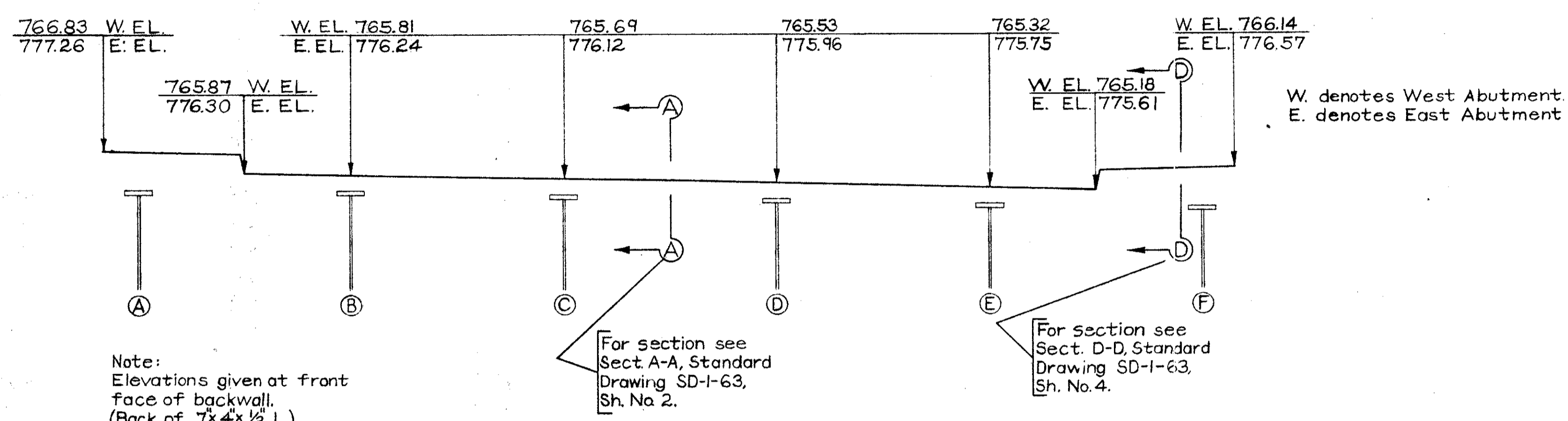
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	CIB		JHO	7/12/64	

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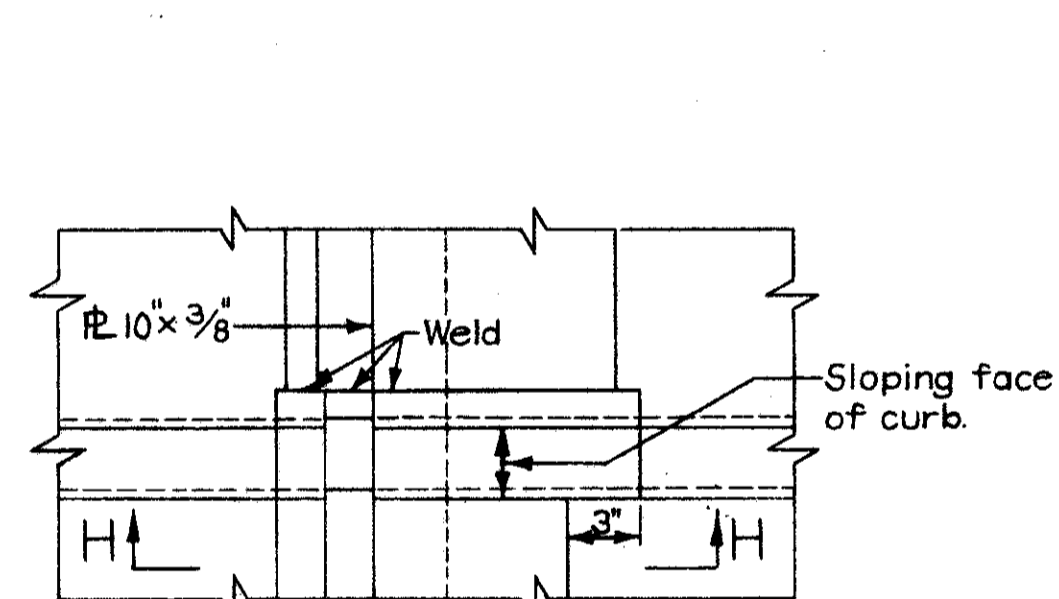
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

437
492

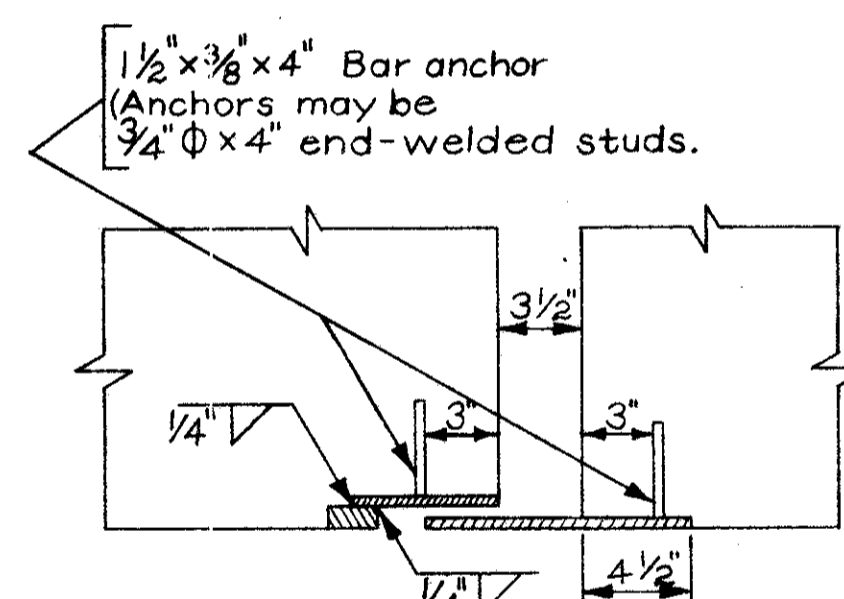
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



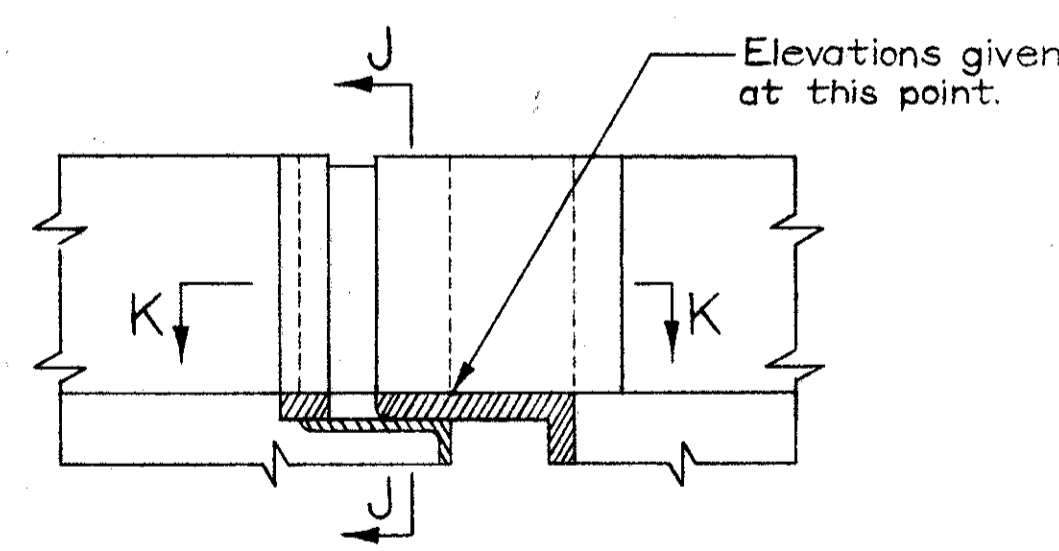
EXPANSION JOINT CROSS SECTION



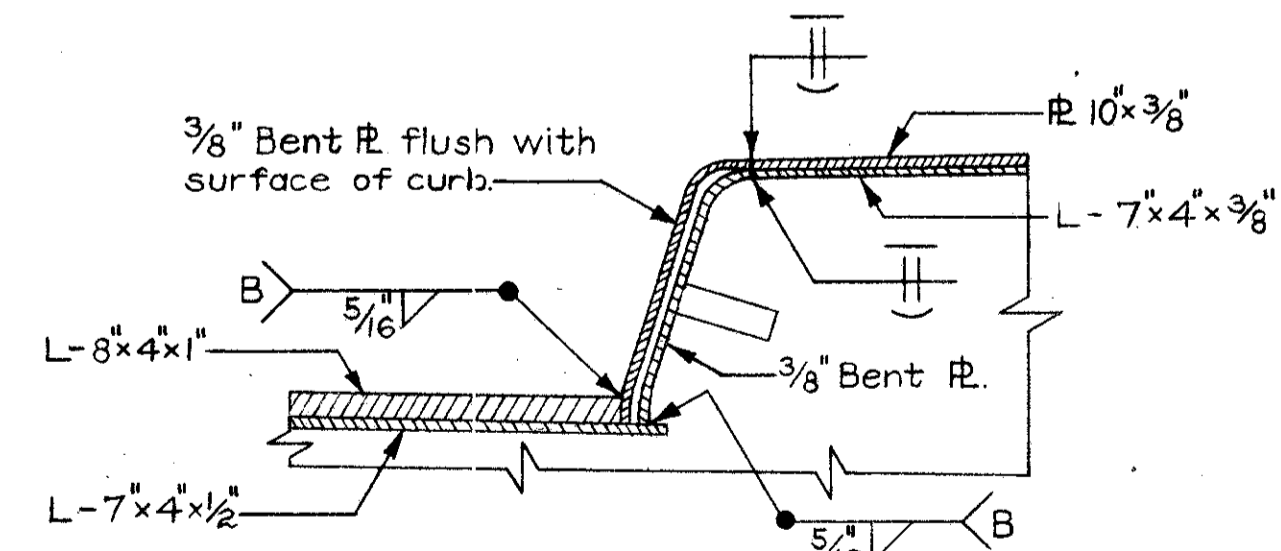
PART PLAN



SECTION K K



SECTION H-H

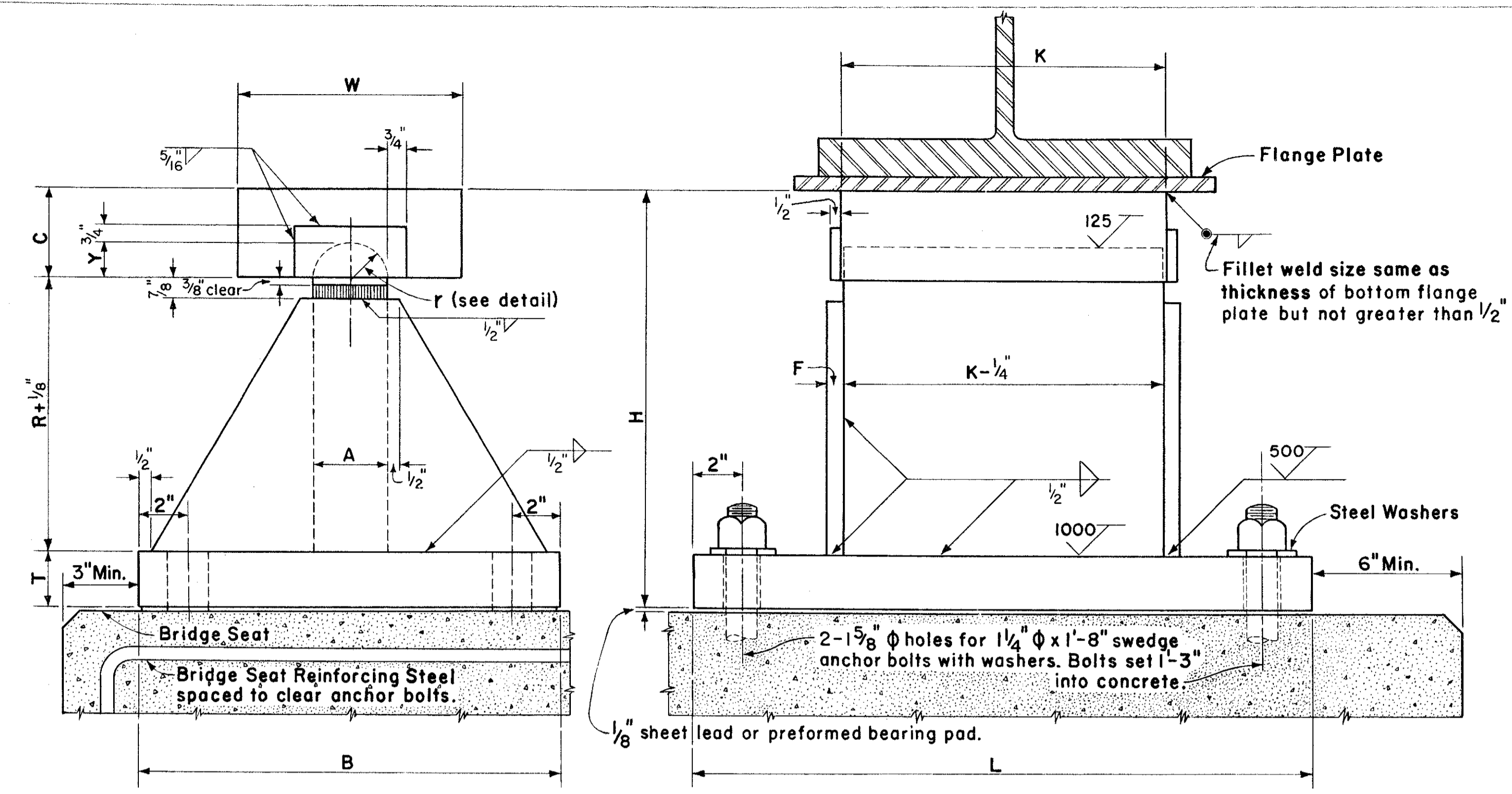


SECTION J J

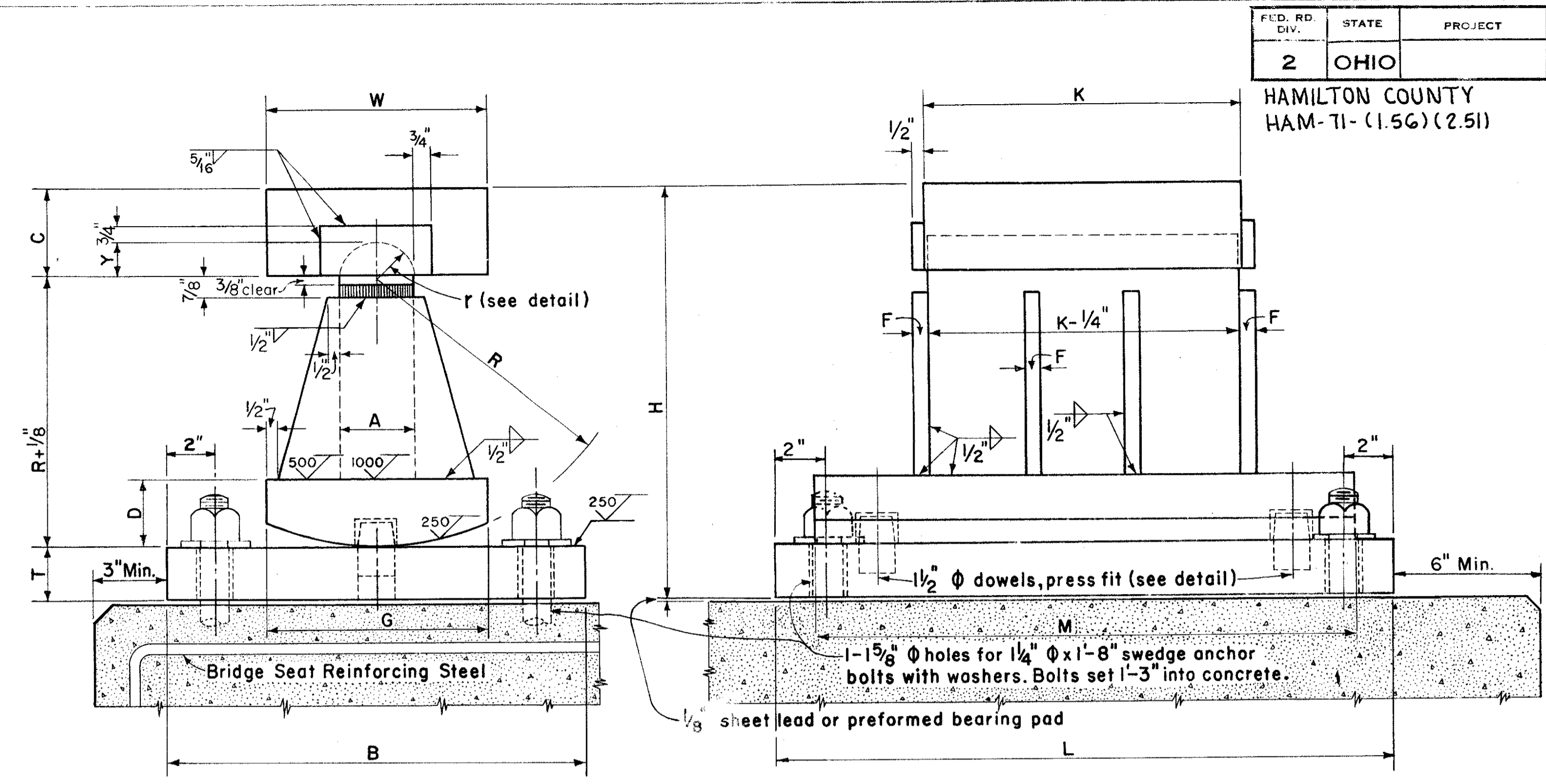
CURB PLATE DETAILS

MICROFILMED
FEB 20 1965

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
STRUCTURAL STEEL DETAILS BRIDGE No. HAM-71-0271					
H&E BRIDGE No 20					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	CIB		JHO	H.A.P. 5-20-65	

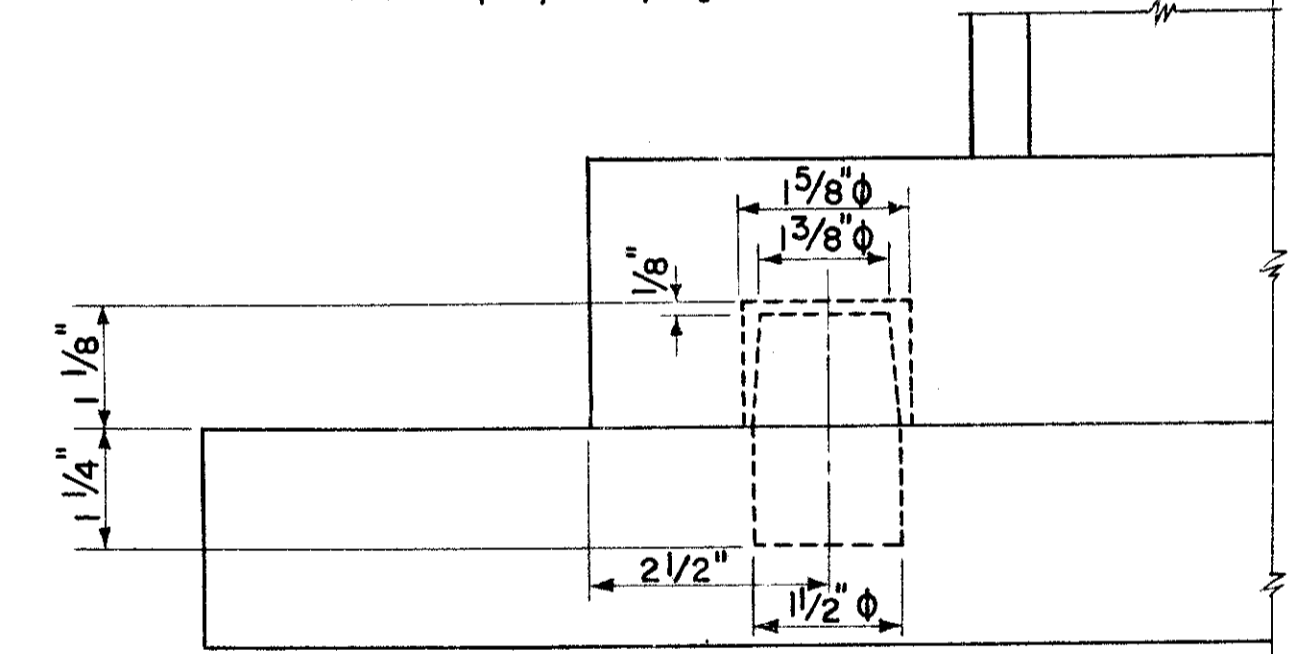


STRUCTURAL STEEL BOLSTER
See Table below for additional dimensions.



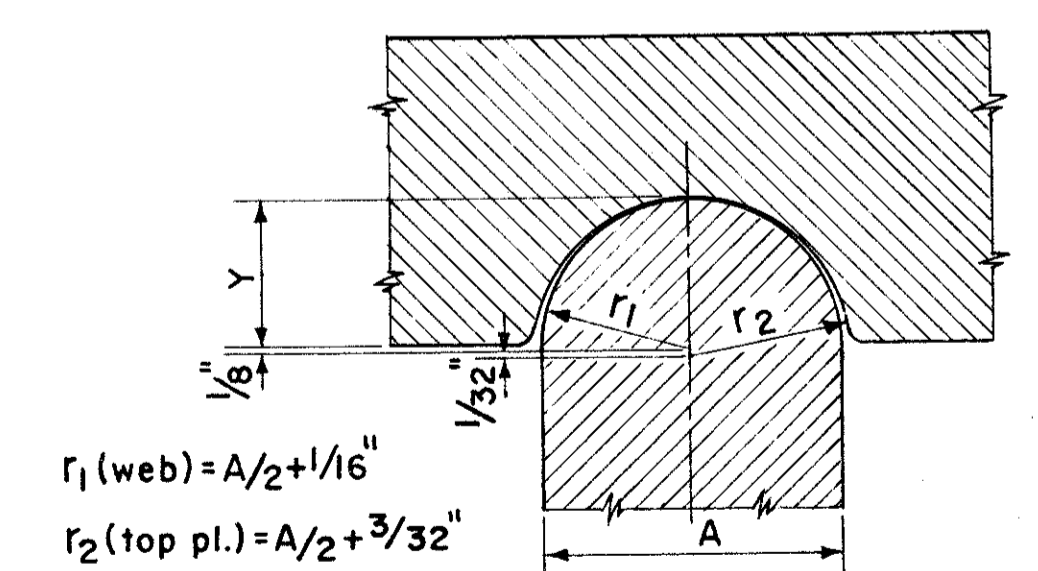
STRUCTURAL STEEL ROCKER
See Table below for additional dimensions.

Design Specifications: This drawing conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated Sept. 1, 1957, together with revisions thereof dated Feb. 21, 1958.



DOWEL DETAIL

BOLSTER No.	ROCKER No.	A	B	C	D	F	G	H	K	L	M	R	T	W	Y
B-325	R-325	4	21	4	3 1/2	3/4	13	20 3/8	15	29	26	13	3 1/4	9	1 15/16
	R-150	3	12	3	2 1/4	1/2	8 1/2	13 3/8	11 1/2	27	19	8 1/2	1 3/4	9	1 7/16



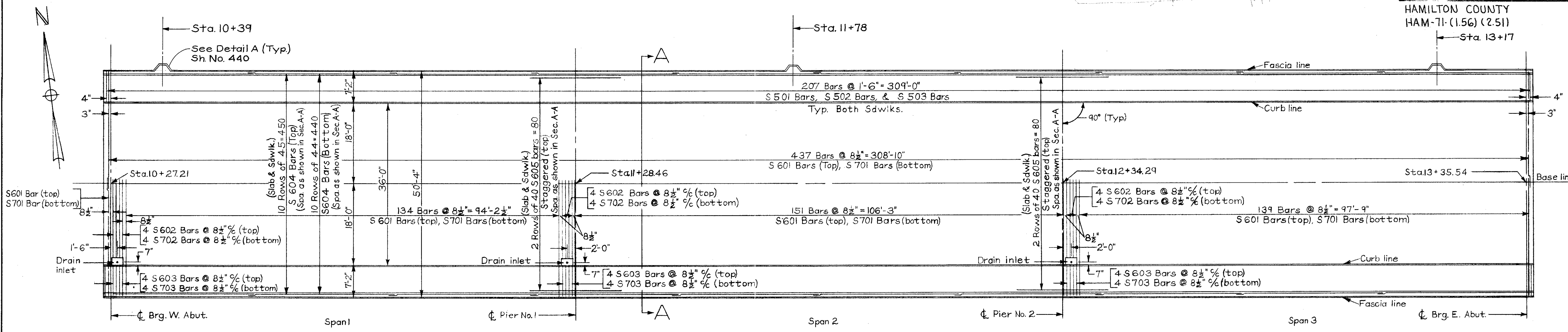
TOP BEARING DETAIL

HAZELET & ERDAL
CONSULTING ENGINEERS
CINCINNATI, OHIO

ROCKERS AND BOLSTERS

DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
			JHO	N.A.F. 5-20-65	

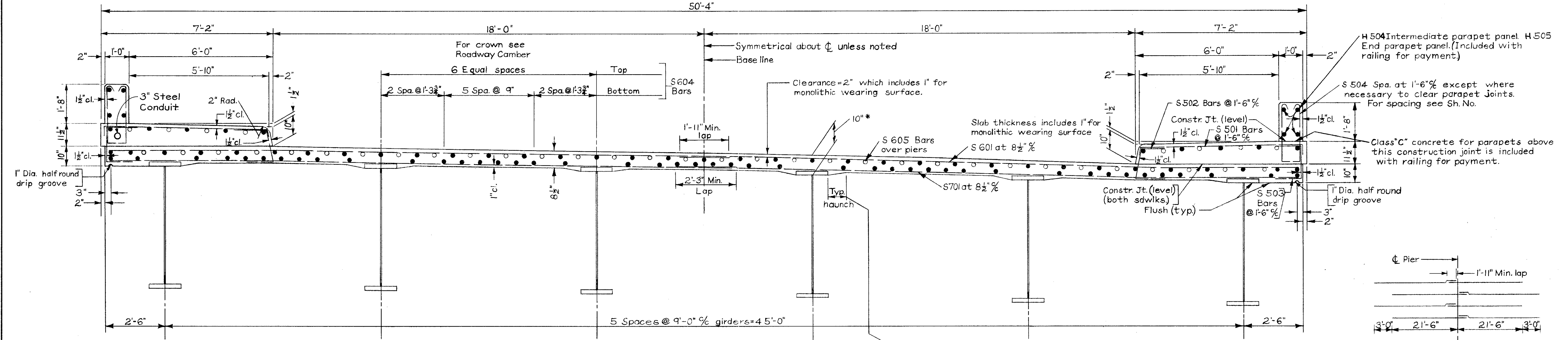
MICROFILMED
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PLAN

Notes:
 For drainage details see Sh. No. 441
 For end finish details see Sh. No. 437
 For lighting details see Sh. No. 216
 For railing details and spacing of bars in parapet, see Sh. No. 440
 Field bend or cut longitudinal bars where necessary to miss inlets.

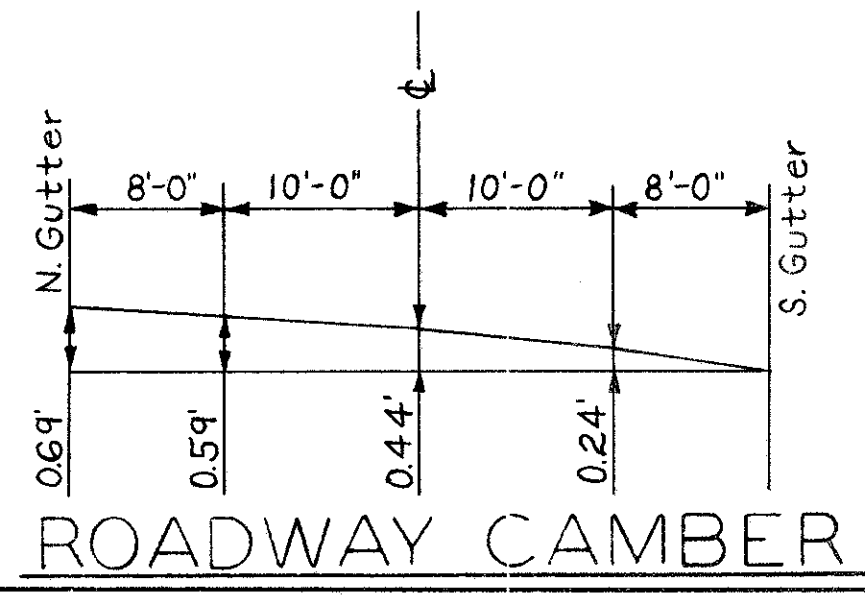
All longitudinal bars S 604 except as otherwise shown Lap S 604 bars 1'-11" minimum.



SECTION A-A

A typical haunch width of 9" shall be used for computing quantity of concrete. However, the haunch width may vary between 6" and 12" provided that the slope shall be not more than 1:4 for a haunch less than 9" in width.

DIAGRAM SHOWING STAGGER OF S 605 BARS OVER PIERS



ROADWAY CAMBER

* This is the nominal dimension. The quantity of deck concrete to be paid for shall be based upon this dimension, even though deviation from it may be necessary because the top flange of the girder may not have the exact camber or conformation required to place it parallel to the finished grade. Deduction shall be made for volume of encased steel plates as per Sec. S-1.2.5 of the Construction and Material Specifications.

REINFORCING STEEL: The S 601 and S 701 Bars may be furnished in pairs of equal lengths (as shown), lapped 30 diameters at the centerline of roadway, or they may be furnished in pairs of different length in order to place the lap beyond a longitudinal construction joint at the centerline of roadway, at the option of the contractor. Determination of the pay quantity will be according to the number and length of bars as shown on Reinforcing Steel List.

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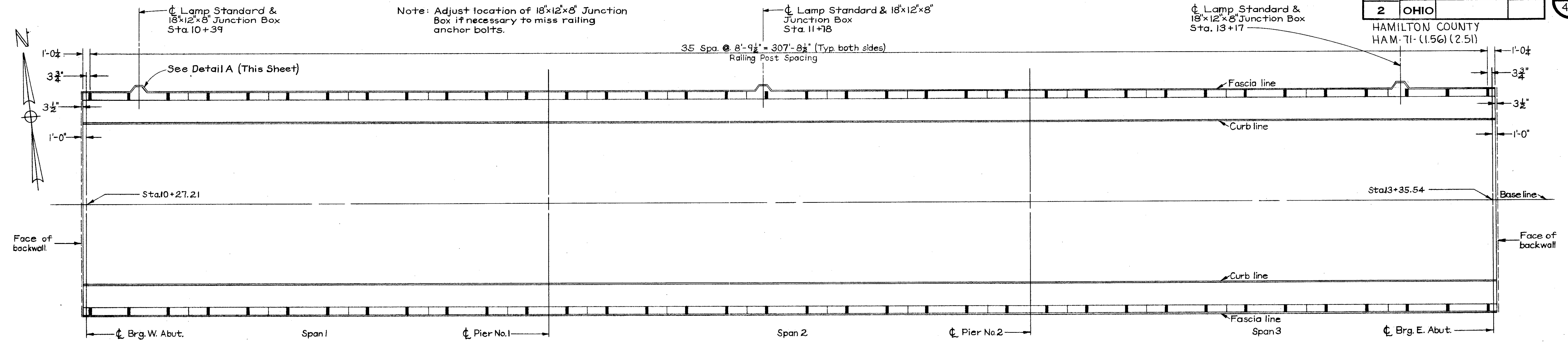
SUPERSTRUCTURE DETAILS
BRIDGE No. HAM-71-0271

H&E BRIDGE No. 20			
DESIGNED	DRAWN	TRACED	CHECKED
CIB	CIB	JHO	N.A.S.
		7/28/64	5-20-65
REVIEWED DATE	REVIEWED		

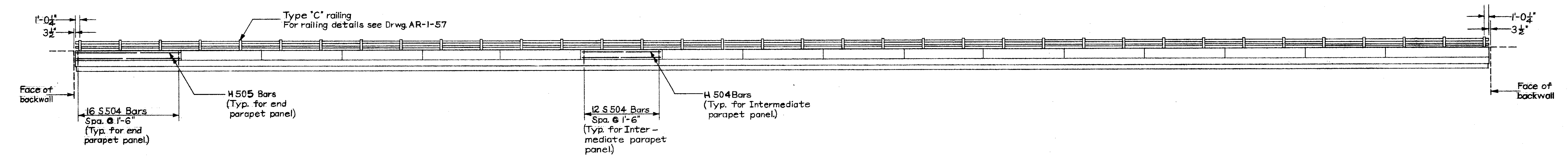
FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

440
492

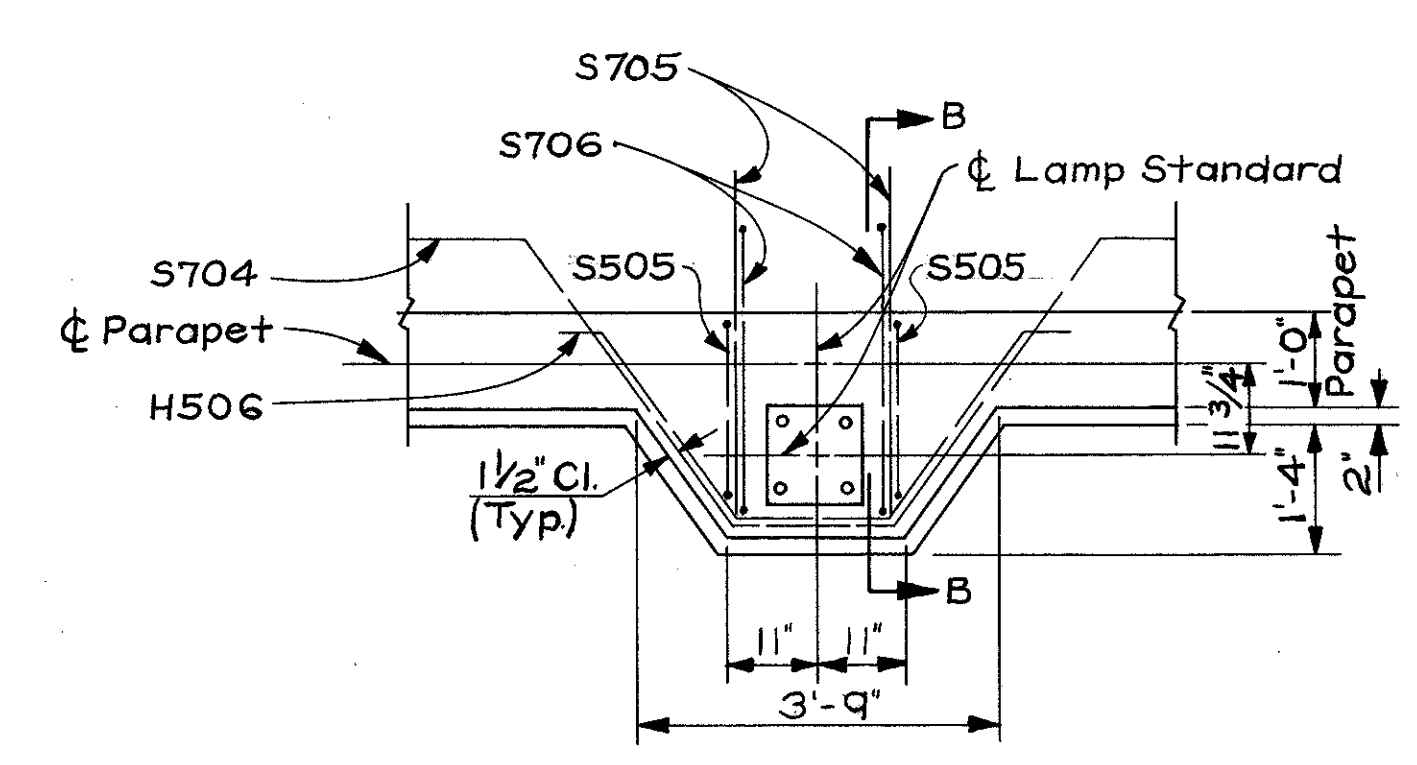
HAMILTON COUNTY
HAM-71-(1.56) (2.51)



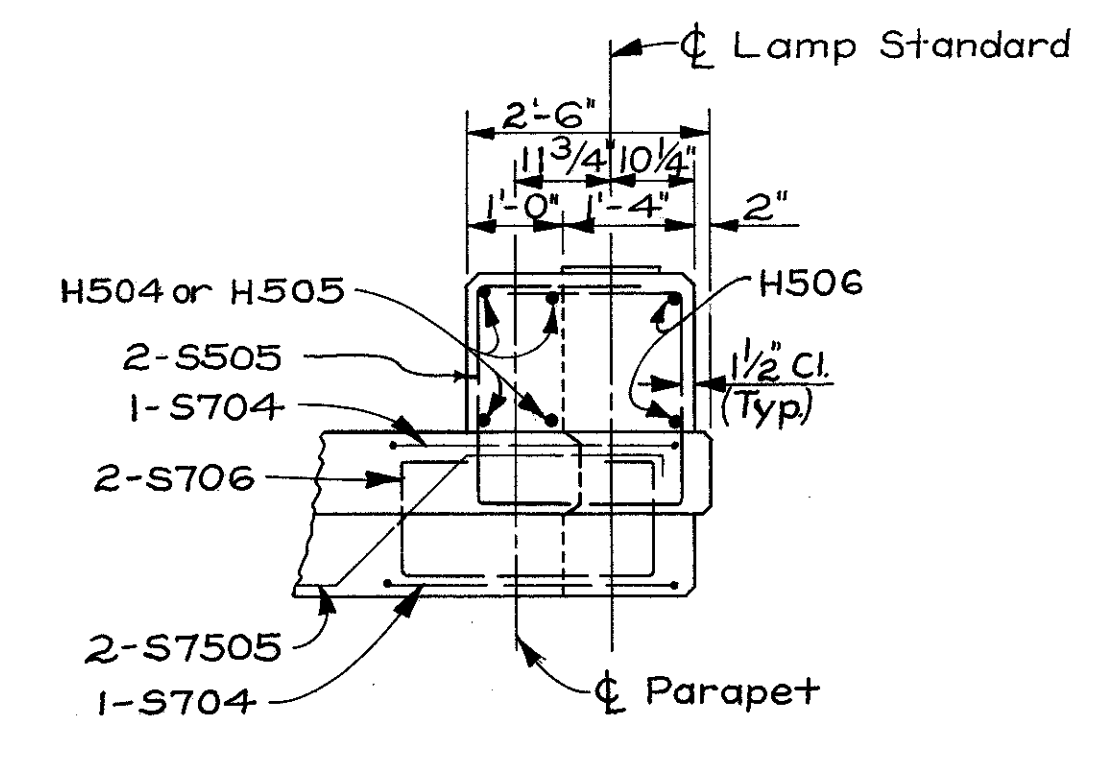
PLAN



ELEVATION



DETAIL A



SECTION B-B

Note: Work this drawing with Lighting Plan, Sheet No. 207.
For typical lighting details see Sh. No. 212.
For conduit expansion at abutments see Sh. No. 214.

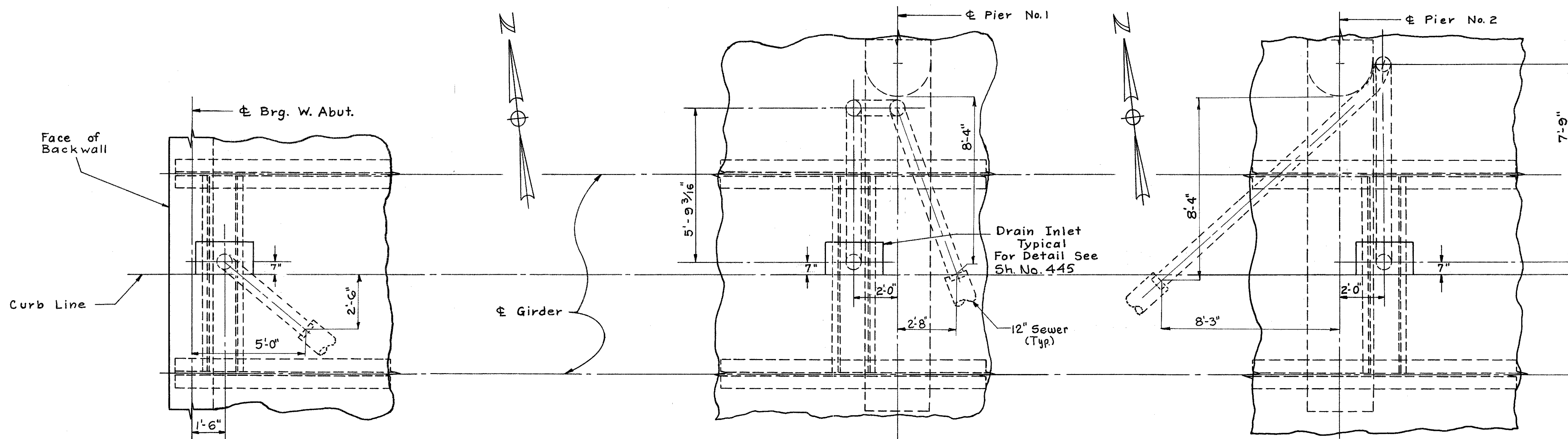
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO				
RAILING & LIGHTING				
BRIDGE No. HAM-71-0271				
H&E BRIDGE No. 20				
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE
	CIB		JWb	H.A.P. 5-20-65

MICROFILMED
FEB 20 1965

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
2	OHIO		

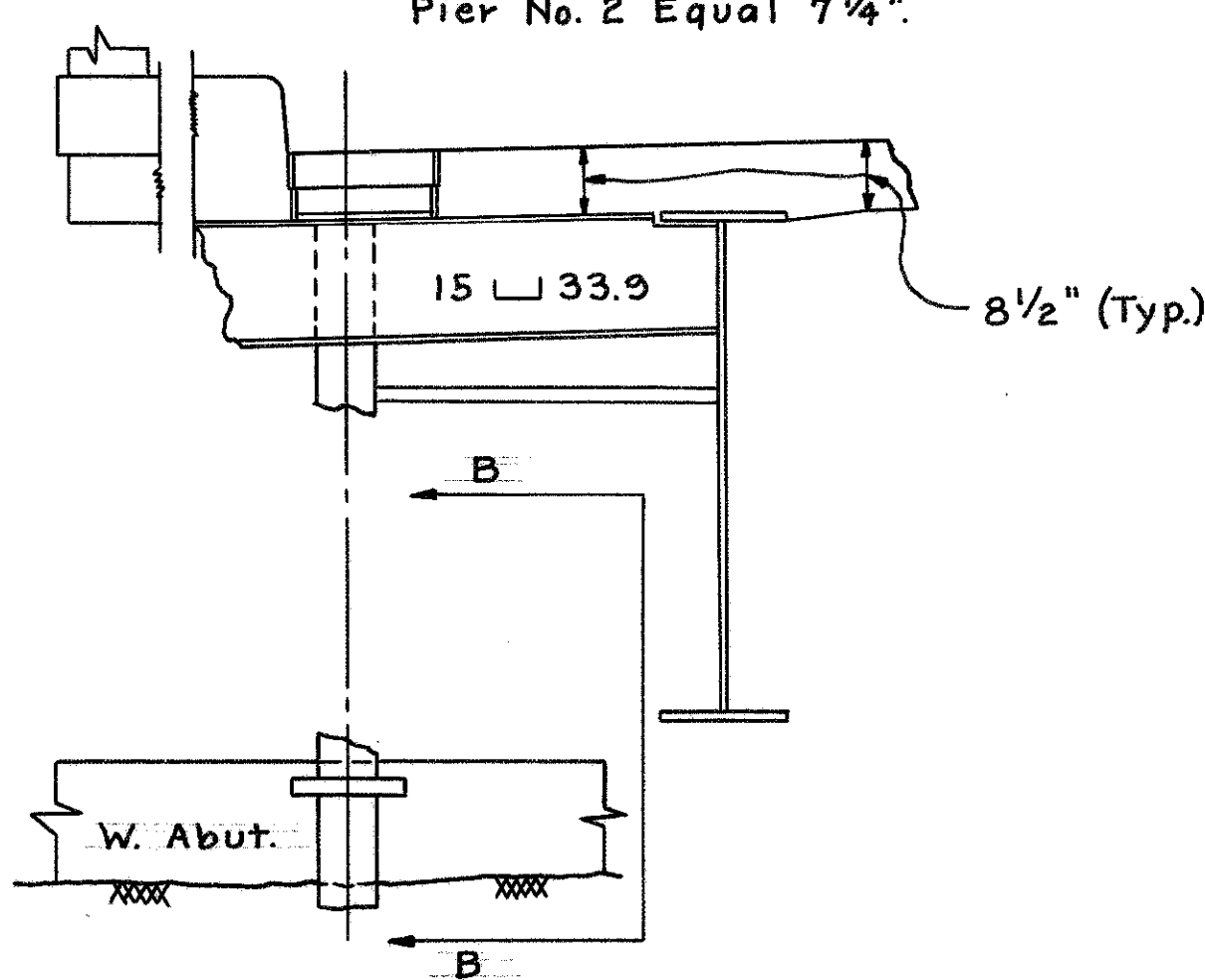
441
492

HAMILTON COUNTY
HAM-71-(1.56)(2.51)



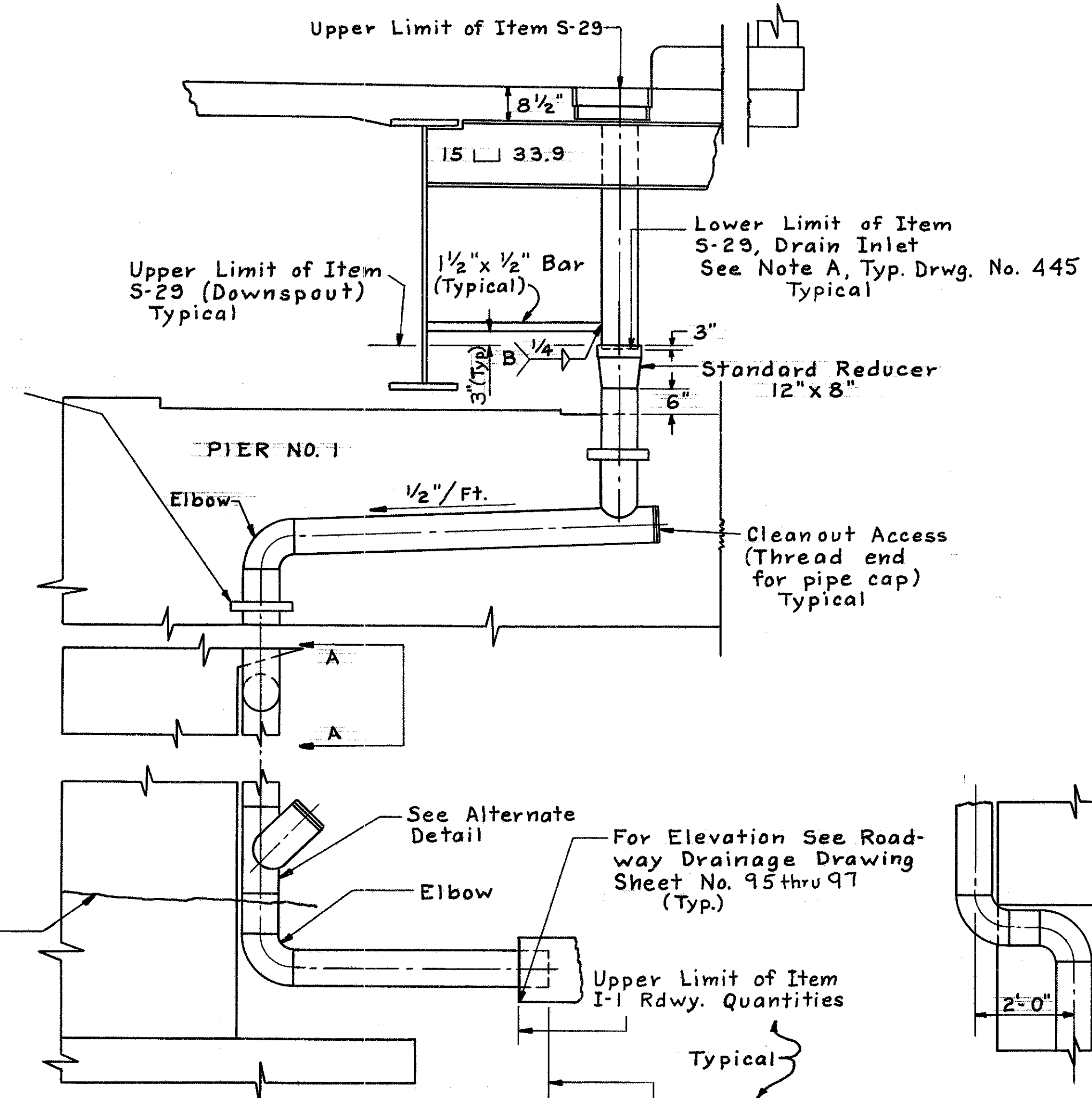
PLAN

Note: Total Depth of Inlet Frame Box at W. Abut., Pier No. 1, and Pier No. 2 Equal 7 1/4".

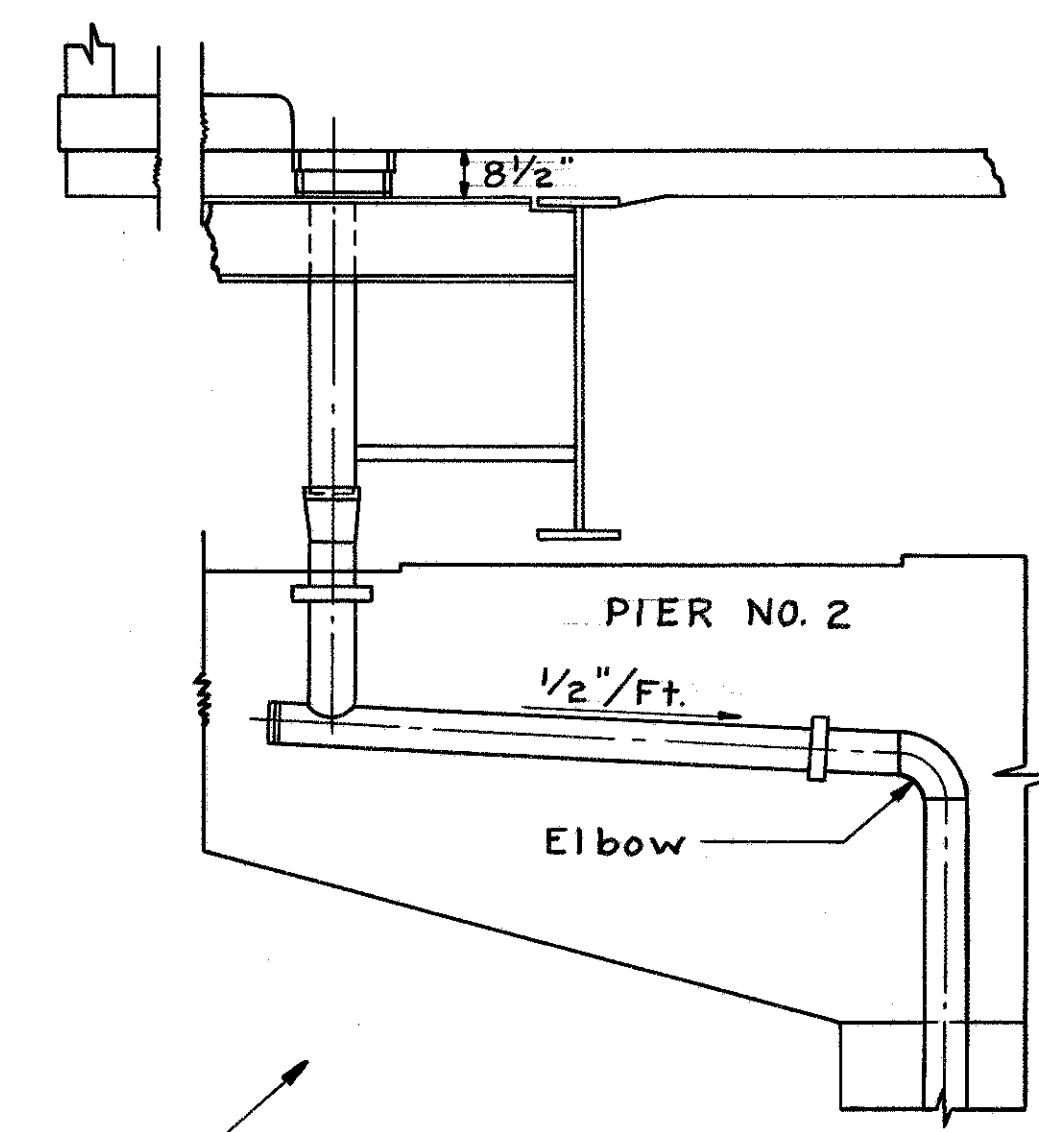


ELEVATION (LOOKING WEST)

Galv. Pipe Strap A (See detail sh. 445 Max. spacing 10'-0")



ELEVATION (LOOKING EAST)



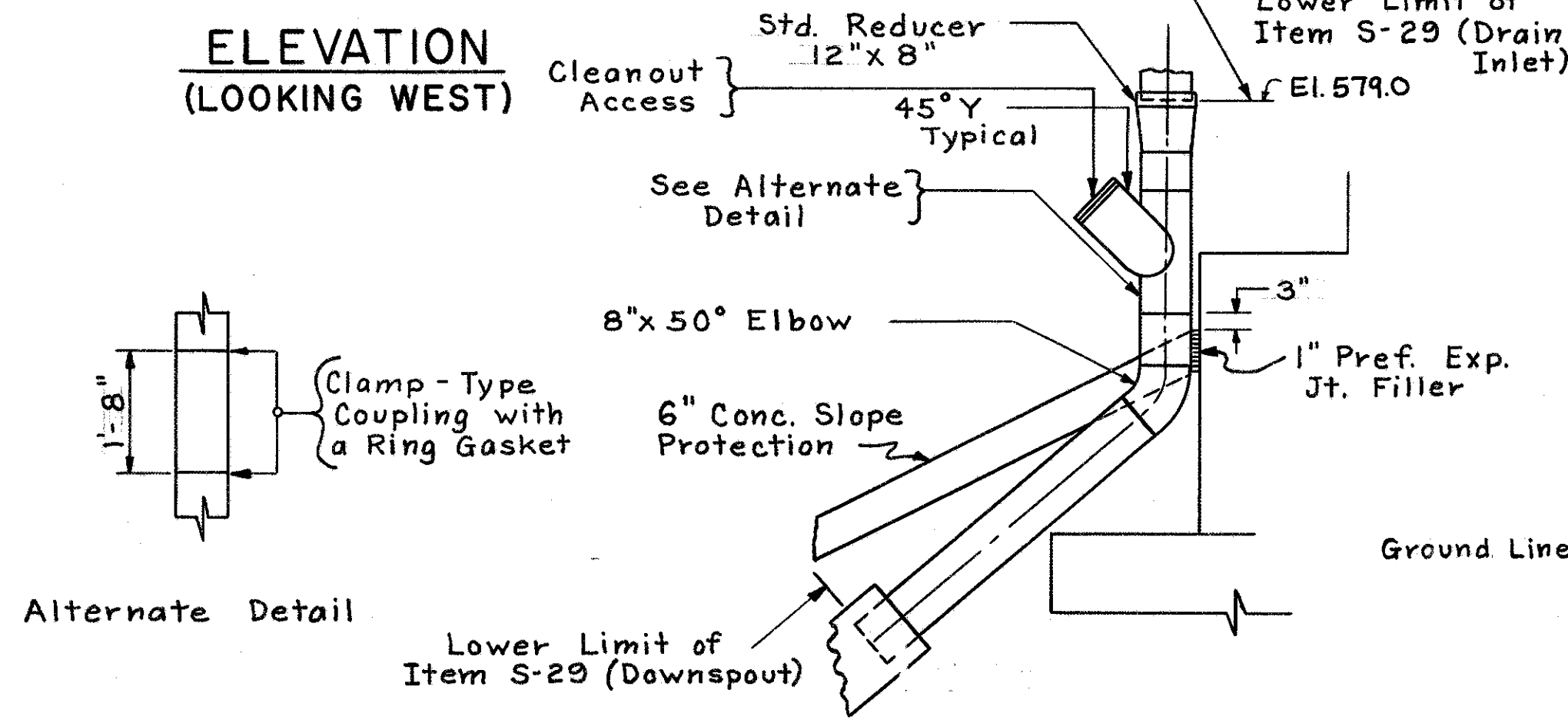
ELEVATION (LOOKING WEST)

All Notes and Dimensions on Pier No. 1 Elevation Apply to Pier No. 2 Elevation

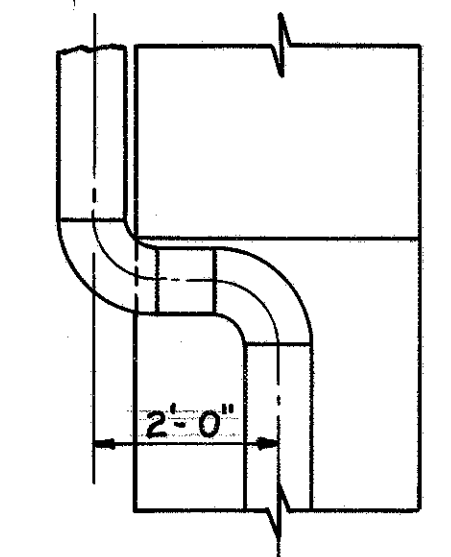
8" Pipe Throughout

Work This Drawing With Typical Drawing Sheet No. 445 "Typical Drainage Details"

MICROFILMED
FEB 20 1985



SECTION B-B



SECTION A-A

HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
DRAINAGE DETAILS					
BRIDGE No. HAM-71-0271					
H&E BRIDGE No. 20					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
LBK.	R.S.	J.H.O.	H.A.Z.	8/20/64	5-20-65

HAMILTON COUNTY
HAM-71-(1.56) (2.51)

SUPERSTRUCTURE				
Mark	Type	No.	Length	Weight
5501	60	414	7'-7"	3274
5502	2	414	3'-4"	1439
5503	1	414	3'-3"	1403
5504	14	360	6'-0"	2253
5505	4	6	10'-0"	63
5601	STR.	862	25'-10"	33447
5602	STR.	12	17'-5"	315
5603	STR.	12	6'-10"	123
5604	STR.	890	32'-9"	43779
5605	STR.	160	24'-0"	5763
5701	STR.	862	26'-0"	45810
5702	STR.	12	17'-7"	431
5703	STR.	12	6'-10"	168
5704	58	6	10'-4"	127
5705	49	6	6'-2"	76
5706	61	6	5'-9"	71
H504	STR.	120	17'-3"	*
H505	STR.	10	22'-9"	*
H506	58	6	8'-1"	*

Total Super. = 138,547 Pounds

WEST ABUTMENT									
Mark	Type	No.	Length	Weight	Mark	Type	No.	Length	Weight
A502	1	40	8'-3"	34	A576	STR.	4	26'-5"	110
A507	1	34	8'-11"	316	A577	STR.	2	6'-8"	14
A508	STR.	40	25'-6"	1064	A578	STR.	2	5'-9"	12
A509	1	34	6'-4"	225	A579	STR.	2	4'-9"	10
A510	1	36	5'-2"	194	A580	STR.	2	3'-10"	8
A517	STR.	12	5'-10"	73	A581	STR.	2	3'-0"	6
A519	STR.	50	7'-11"	413					
A520	STR.	16	3'-7"	60	A606	1	4	19'-9"	119
A521	20	2	9'-5"	20	A607	1	2	8'-2"	25
A531	32	38	6'-4"	251	A608	1	2	7'-5"	22
A533	18	6	10'-0"	63	A609	STR.	4	9'-10"	60
A534	18	7	10'-2"	74	A610	7	36	19'-6"	1054
A535	18	6	10'-4"	64	A613	8	6	16'-1"	145
A536	18	6	10'-6"	66	A614	8	7	16'-3"	171
A537	18	9	10'-8"	100	A615	8	6	16'-5"	148
A538	STR.	4	20'-7"	86	A616	8	6	16'-7"	149
A539	STR.	6	18'-9"	117	A617	1	6	10'-2"	92
A540	STR.	2	22'-9"	47	A618	1	34	10'-10"	553
A541	STR.	6	26'-7"	166	A621	STR.	5	11'-3"	84
A542	STR.	8	2'-8"	22	A622	1	23	23'-4"	806
A543	STR.	2	2'-0"	6	A623	1	24	22'-0"	793
A544	STR.	2	2'-10"	6	A624	STR.	4	10'-7"	64
A545	STR.	3	3'-3"	7	A625	8	9	16'-9"	226
A546	STR.	2	3'-10"	8					
A547	STR.	2	4'-5"	9	A801	STR.	18	28'-10"	1386
A548	STR.	2	5'-0"	10	A804	46	4	23'-1"	247
A549	STR.	2	5'-7"	12	A806	46	4	20'-1"	214
A550	STR.	6	6'-2"	39	A807	46	2	17'-1"	91
A551	STR.	2	6'-3"	13	A808	46	2	20'-11"	112
A552	STR.	2	6'-4"	13					
A553	STR.	2	6'-5"	13	H502	STR.	4	26'-7"	*
A554	STR.	2	6'-6"	14	H503	STR.	4	26'-5"	*
A555	STR.	2	6'-7"	14					
A556	STR.	2	6'-8"	14					
A557	STR.	2	6'-9"	14					
A558	STR.	2	6'-10"	14					
A559	STR.	2	6'-11"	14					
A560	STR.	2	7'-0"	15					
A561	STR.	2	7'-1"	15					
A562	STR.	2	7'-2"	15					
A563	STR.	2	7'-3"	15					
A564	STR.	2	7'-4"	15					
A565	STR.	2	7'-5"	15					
A566	STR.	2	7'-6"	16					
A567	STR.	2	7'-7"	16					
A568	STR.	2	7'-8"	16					
A569	STR.	2	7'-9"	16					
A570	STR.	2	7'-10"	16					
A571	20	2	8'-1"	17					
A572	STR.	4	20'-5"	85					
A573	STR.	6	18'-7"	116					
A574	STR.	2	20'-10"	43					
A575	STR.	2	22'-11"	48					

Total W. Abut. = 10,875 Pounds

EAST ABUTMENT				
Mark	Type	No.	Length	Weight
A501	18	9	11'-1"	104
A502	1	38	8'-3"	327
A503	18	6	11'-0"	69
A504	18	6	10'-10"	68
A505	18	7	10'-8"	78
A506	18	6	10'-6"	66
A507	1	12	8'-11"	112
A508	STR.	40	25'-6"	1064
A509	1	34	6'-4"	225
A510	1	36	5'-2"	194
A511	STR.	8	10'-2"	85
A512	STR.	12	6'-4"	79
A513	STR.	32	8'-2"	273
A514	STR.	2	13'-2"	27
A515	STR.	2	11'-5"	24
A516	STR.	8	16'-2"	135
A517	STR.	12	5'-10"	73
A518	STR.	6	5'-11"	37
A519	STR.	10	7'-11"	83
A520	STR.	16	3'-7"	60
A522	STR.	4	5'-9"	24
A523	STR.	4	3'-4"	22
A524	STR.	4	5'-0"	21
A525	STR.	4	4'-8"	19
A526	STR.	4	4'-3"	18
A527	STR.	4	3'-11"	16
A528	STR.	4	3'-6"	15
A529	STR.	4	3'-2"	13
A530	20	4	10'-5"	43
A531	32	24	6'-4"	159
A532	56	16	9'-5"	157
A601	8	9	17'-3"	233
A602	8	6	17'-2"	155
A603	8	6	17'-0"	153
A604	8	7	16'-10"	177
A605	8	6	16'-8"	150
A606	1	4	19'-9"	119
A607	1	2	8'-2"	25
A608	1	2	7'-5"	22
A609	STR.	4	9'-10"	59
A610	7	36	19'-6"	1054
A611	1	7	24'-0"	252
A612	STR.	10	11'-10"	178
A617	1	4	10'-2"	61
A618	1	12	10'-10"	195
A619	1	7	22'-8"	238
A620	STR.	10	11'-2"	168
A801	STR.	18	28'-10"	1386
A802	STR.	4	13'-0"	139
A803	STR.	8	9'-6"	203
A805	56	4	7'-9"	83
H501	STR.	8	16'-2"	*

Total E. Abut. = 8,740 Pounds

PIERS						
Mark	Type	No.			Length	Weight
		Pier 1	Pier 2	Total		
P501	STR.	4	4	8	22'-4"	186
P502	STR.	2	2	4	34'-0"	142
P503	STR.	8	8	16	24'-10"	414
P504	1	184	184	368	10'-0"	3838
P505	1	22	22	44	10'-8"	490
P506	1	102	102	204	15'-2"	3227
P507	1	38	38	76	14'-6"	1149
P508	1	22	22	44	9'-6"	436
P509	1	36	36	72	6'-5"	482
P510	1	36	36	72	5'-5"	407
P601	STR.	-	18	18	18'-6"	500
P602	STR.	9	10	19	34'-6"	985
P603	20	8	8	16	15'-9"	379
P604	STR.	-	18	18	16'-6"	446
P605	STR.	24	32	56	19'-4"	1626
P606	6	24	32	56	7'-10"	659
P1001	43	-	52	52	21'-4"	4773
P1002	43	28	32	60	37'-4"	9639
P1003	STR.	8	8	16	23'-0"	1584
P1004	STR.	8	8	16	20'-0"	1377
P1005	62	4	4	8	43'-10"	1509
P1006	62	4	4	8	41'-4"	1423
P1007	43	44	-	44	19'-4"	3660
P1101	44	60	60	120	8'-11"	5685
P1102	STR.	-	60	60	26'-1"	8315
P1103	18	4	4	8	20'-8"	878
P1104	STR.	4	4	8	34'-7"	1470
P1105	STR.	60	-	60	21'-4"	6801
P1106	18	4	4	8	38'-3"	1626
P1107	STR.	4	4	8	17'-0"	723

Total Piers = 64,829 Pounds

SUPERSTRUCTURE									
Mark	a	b	c	d	e	f	g	h	i
5501	0'-6"	6'-9"	0'-1 1/2"	0'-6"	0'-6"				
5502	1'-4"	0'-3"	1'-0"	1'-3 3/4"	1'-0"				
5503	1'-6"	1'-0"							
5504	0'-9"	2'-4"							
5505	2'-4"	1'-10"	1'-10"	2'-4"	2'-1"	2'-9"	2'-9"	3'-9"	0'-9"
5704	1'-8"	2'-6"	3'-9"	2'-6"	0'-9"	2'-9"	2'-9"	3'-9"	0'-9"
5705	2'-5"	1'-10 1/2"	1'-3 3/4"	1'-4"	0'-6"	1'-9"			
5706	0'-5"	1'-4"	2'-11"						
H506	1'-8"	1'-9 1/2"	2'-7 1/2"	1'-9 1/2"	0'-9"	1'-11"	1'-11"	2'-7 1/2"	0'-9"

PIERS					
Mark	a	b	c	d	e
P504	1'-10 1/2"	4'-2"			
P505	1'-10 1/2"	4'-6"			
P506	1'-10 1/2"	6'-9"			
P507	1'-10 1/2"	6'-5"			
P508	1'-10 1/2"	3'-11"			
P509	2'-8"	2'-0"			
P510	2'-8"	1'-6"			
P603	0'-8"	13'-9"	2'-0"	1'-10 1/2"	
P606	3'-10"	2'-0"		Radius = 1'-2"	
P1001	18'-6"				
P1002	34'-6"				
P1005	4'-9"	4'-9"	30'-4"	6'-9"	
P1006	5'-3 1/2"	5'-3 1/2"	26'-4"	7'-6"	
P1007	16'-6"				
P1101	7'-4"				
P1103	4'-0"	17'-0"			
P1106	4'-0"	34'-7"			

EAST ABUTMENT					
Mark	a	b	c	d	e
A501	1'-7"	9'-8"			
A502	5'-4"	1'-7"			
A503	1'-7"	9'-7"			
A504	1'-7"	9'-5"			
A505	1'-7"	9'-3"			
A506	1'-7"	9'-1"			
A507	6'-0"	1'-7"			
A509	3'-5"	1'-7"			
A510	3'-5"	1'-0"			
A530	3'-0 1/2"	2'-0"	8'-5"	7'-10"	
A531	0'-8"	2'-6"			
A532	4'-5"	1'-7"	6'-3"	1'-7"	4'-5"
A601	2'-7"	5'-4"	9'-8"		
A602	2'-7"	5'-4"	9'-7"		
A603	2'-7"	5'-4"	9'-5"		
A604	2'-7"	5'-4"	9'-3"		
A605	2'-7"	5'-4"	9'-1"		
A606	1'-5"	9'-4"			
A607	0'-6"	1'-0"			
A608	5'-9"	1'-0"			
A610	0'-11"	8'-7"	1'-11"	1'-5"	7'-3"
A611	1'-2"	11'-7"			
A617	5'-4"	2'-7"			
A618	6'-0"	2'-7"			
A619	1'-2"	10'-11"			
A805	1'-11"	2'-6"	2'-9"	2'-6"	1'-11"

WEST ABUTMENT				
Mark	a			

FED. RD. DIV.	STATE	PROJECT	FISCAL YEAR
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HAMILTON COUNTY
HAM-71-(1.56) (2.51)


Note 1

REFERENCE shall be made to Standard Drawings AR-1-57 "Revised 4-2-62", FSB-1-62 "Revised 1-15-63", SD-1-63 dated 11-12-63 Sheet Nos. 2 and 4, Supplemental Specification No. S-307 "Rev. 10-1-64", Supplemental Specification No. I-129 "Revised 4-5-61", and Supplemental Specification No. S-101 dated 7-12-62.

Note 2

PILES: Since the structures of this project are to be constructed in a metropolitan area where there are numerous areas in which buildings have been dismantled and the existing basements filled with boulders, gravel, bricks, and other random debris, the Contractor shall use augering, spudding or whatever means are necessary to permit the piles to be driven without damaging them whenever the above conditions are encountered.

Note 3

WELDING shall be Class "A" except as shown. Any welds shown as field welds may, at the option of the Contractor, be made in the shop. Class "B" welds are shown thus: 

Note 4

POROUS BACKFILL (where called for in the plans) shall be 2ft. thick and shall extend up to the underside of the approach slab or side-walk unless otherwise noted.

Note 5

DECK PLACING PROCEDURE: In placing the deck concrete, construction joints will be permitted parallel to the transverse reinforcing steel and near the middle of any span. Because of the flow of curing water from the surface of previously placed concrete, the sequence of pours shall be upgrade, starting at the lowest point or points in the grade line.

Note 6

UTILITY LINES: All labor and expense involved in relocating (installing) the affected utility lines shall be borne by the owners. The Contractor and Owners are requested to cooperate by arranging their work in such a manner that inconvenience to either will be held to a minimum.

Note 7

DESIGN SPECIFICATIONS: This structure conforms to the requirements of "Design Specifications for Highway Structures" of the State of Ohio, Department of Highways, dated 9-1-57, together with current revisions thereof.

Note 8

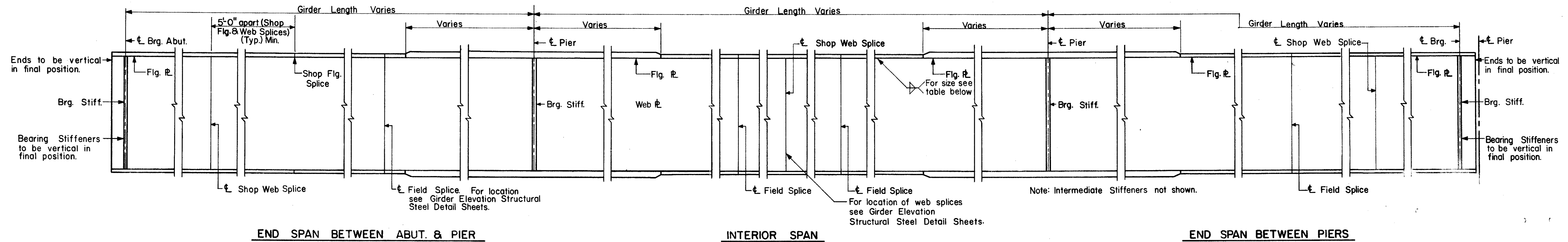
HIGH-STRENGTH STEEL BOLTS: Item S-7.10, paragraph two (2), shall be completely revised and the last sentence of paragraph four (4) revised to read as follows:

"In the final assembly of the parts to be bolted, drift pins shall be placed in a sufficient number of holes (not less than 25 percent for field erection) to provide and maintain accurate alignment of holes and parts, and sufficient bolts shall be installed and brought to a snug tight condition to bridge the parts into complete contact. Bolts shall then be installed in any remaining open holes and tightened to a snug tight fit, after which all bolts shall be tightened completely by calibrated wrenches or by the turn-of-nut method. Drift pins shall then be replaced with bolts, tightened in the same manner.

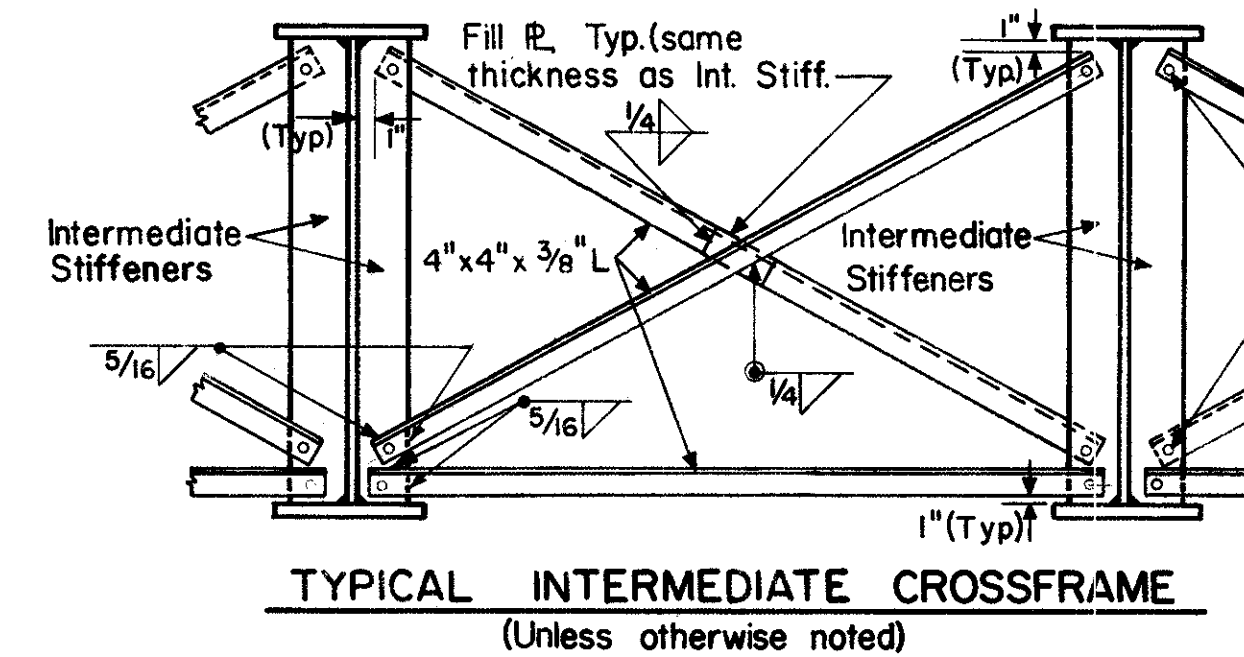
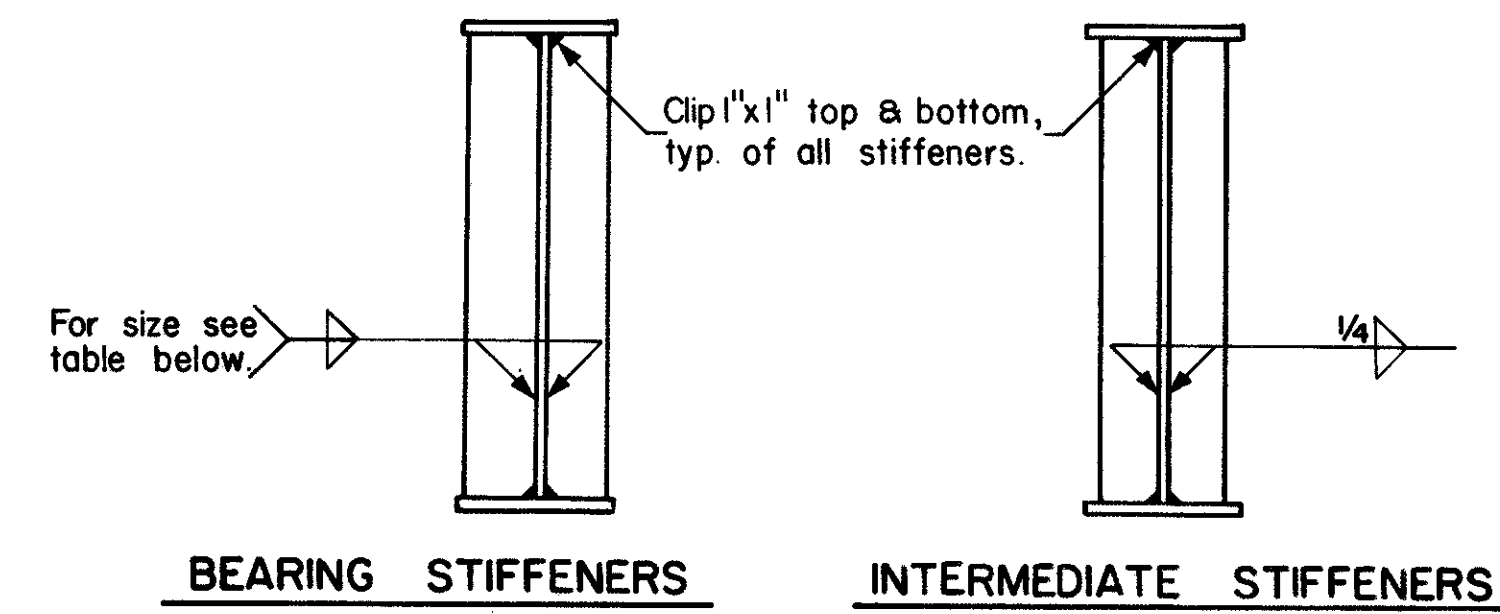
"Bolt lengths determined by the use of Table No.1 shall be adjusted to the next 1/4 inch length increment."

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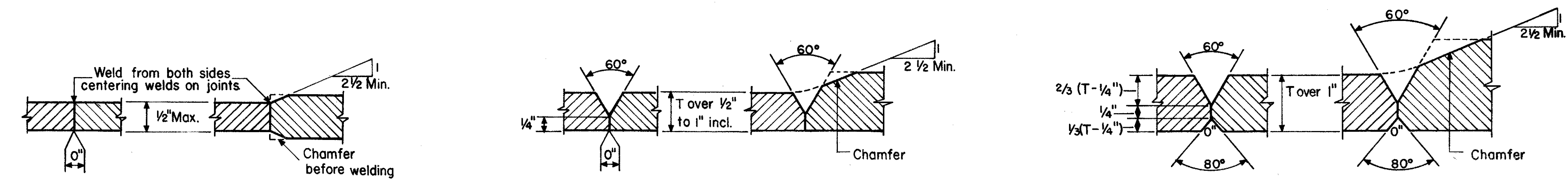
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
GENERAL NOTES					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	MJE		JHO	H.A.B. 5-20-65	
	8-5-64		8-11-64		



TYPICAL PLATE GIRDER ELEVATIONS



NOTE A:
Provide $1\frac{3}{16}$ " holes for $\frac{3}{4}$ " bolts in crossframe angles and stiffener plates, which are to be field connected before welding. If $\frac{3}{4}$ " bolts are to be left in place, they shall be tack welded.



JOINT PREPARATION FOR SUBMERGED ARC WELDMENTS

All of the above full penetration welds shall be back-gouged and welded after welding for side.
Butt welds on beam and girder flange plates shall be ground flush, the finish grinding being parallel to the direction of stress.

Plate Thickness	Fillet Weld Size
Up to $\frac{3}{4}$ "	$\frac{1}{4}$ "
Over $\frac{3}{4}$ " to $1\frac{1}{2}$ "	$\frac{5}{16}$ "
Over $1\frac{1}{2}$ " to $2\frac{1}{4}$ "	$\frac{3}{8}$ "
Over $2\frac{1}{4}$ " to 6"	$\frac{1}{2}$ "

Plate Thickness refers to the thickness of the thicker part joined.

NOTES:

Bearing Stiffeners over abutments and piers shall be grooved and fully butt-welded to the lower flange and fitted in close contact without welding to the upper flange.

Intermediate Stiffeners shall have contact bearing with the compression flange, but may have a clearance of not more than $\frac{1}{8}$ " from the tension flange. In shop painting care shall be taken to make certain that paint is forced

through from one side to the other of the $\frac{1}{8}$ " opening.

For examination of welds for all plate girder spans see Supplemental Specification No. S-307.

The contractor shall submit to the Director, for approval, 3 prints showing erection procedure for the plate girders.

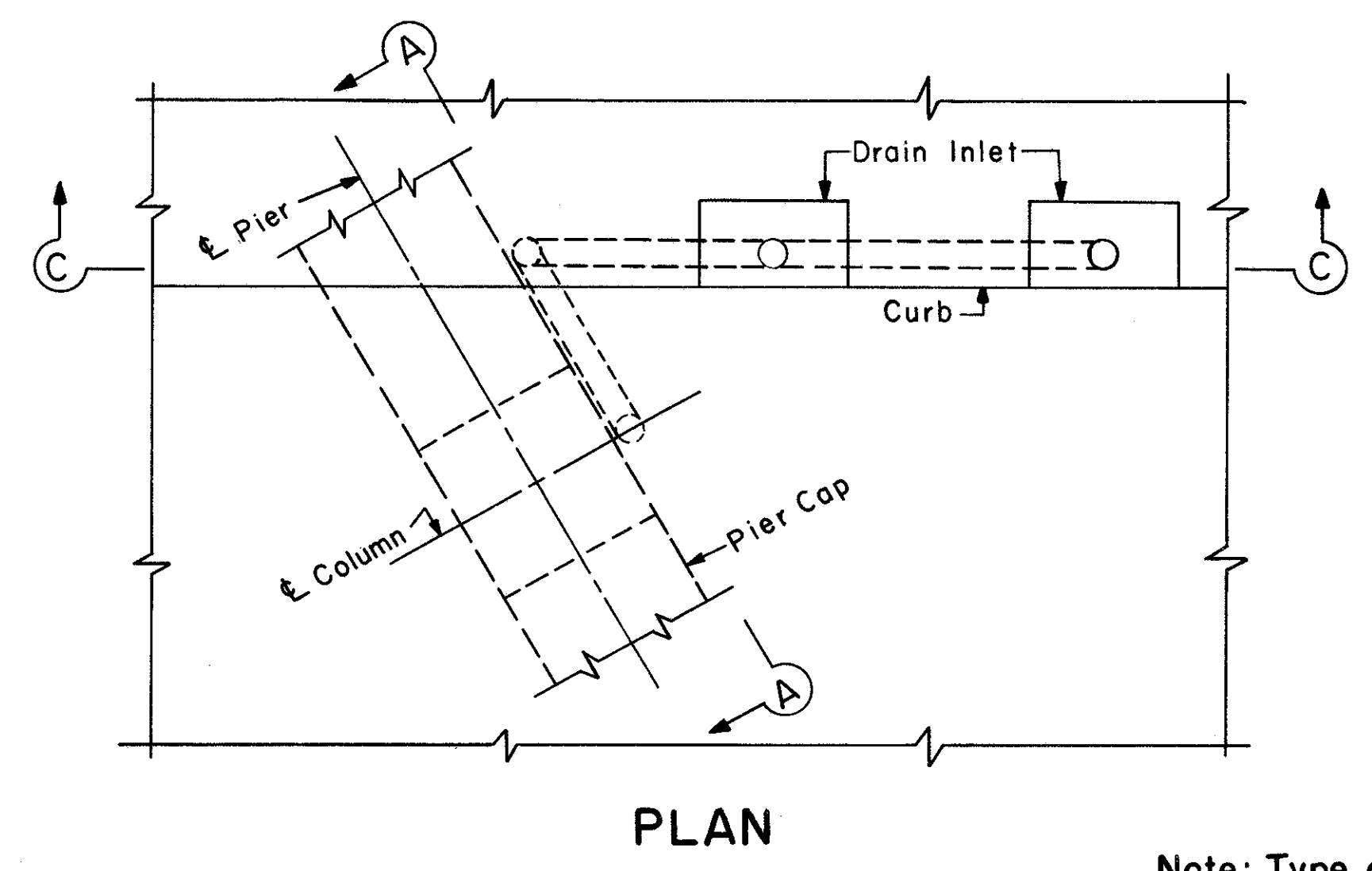
Structural Steel - ASTM A36 - basic unit stress 20,000psi

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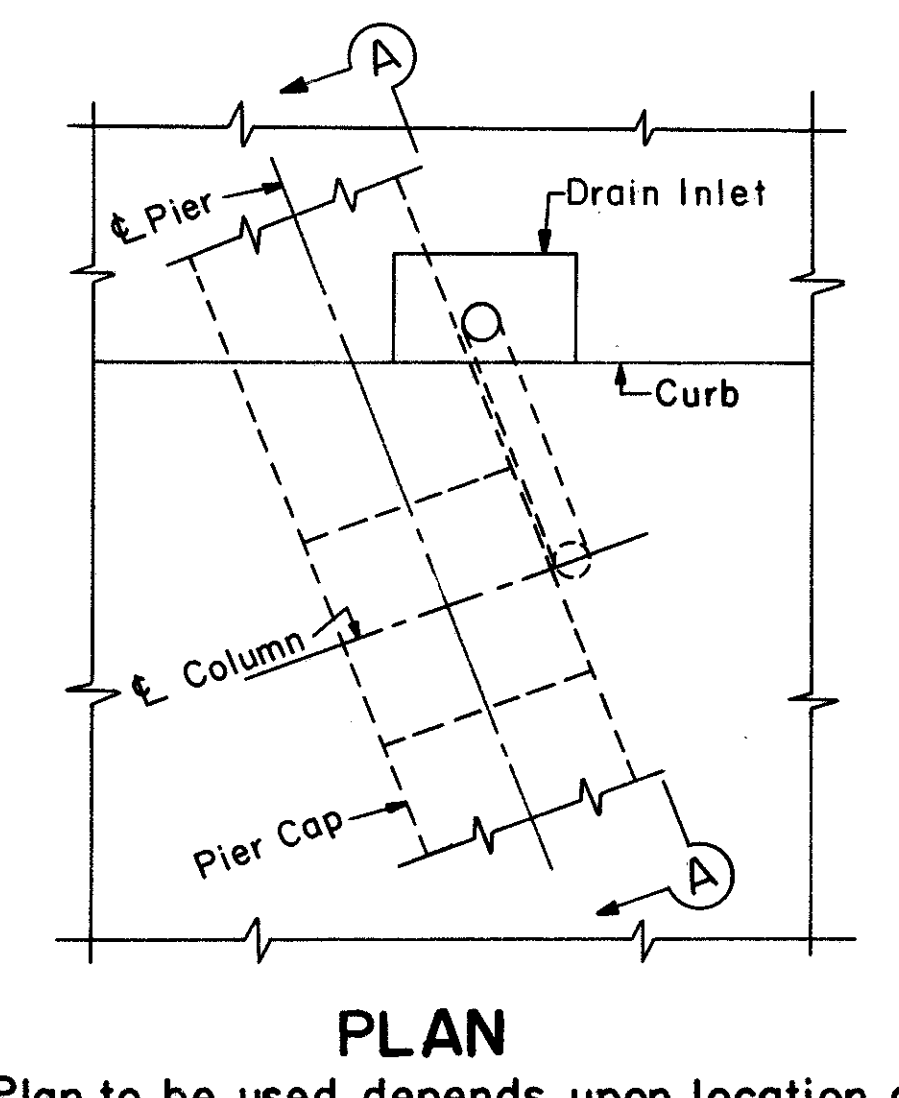
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TYPICAL STRUCTURAL STEEL DETAILS					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
	M.J.E.		J.H.O.	H.A.Z. 5-20-65	

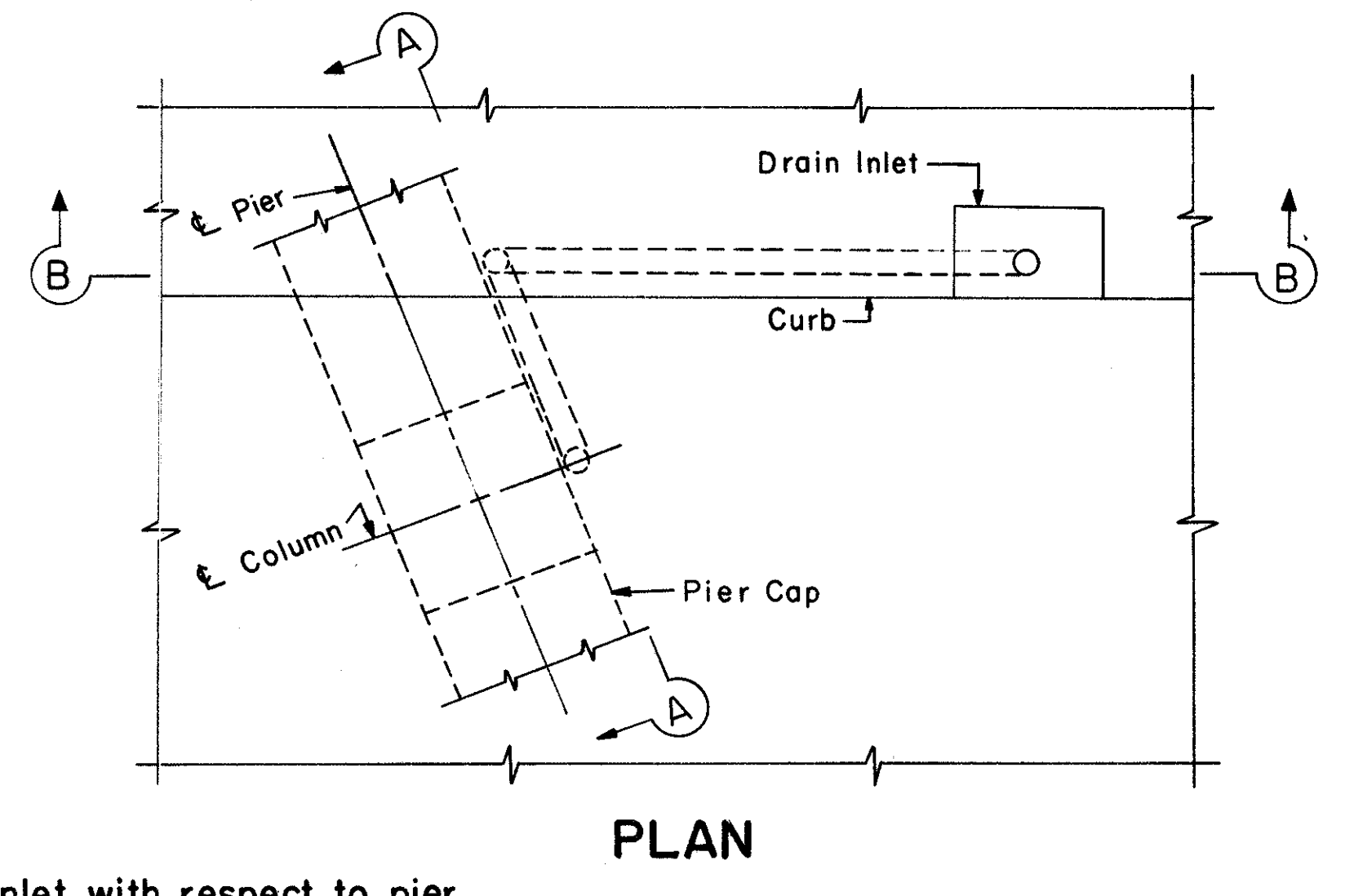
HAMILTON COUNTY
HAM-71-(1.5G)(2.51)
Upper Limit of Item S-29



PLAN



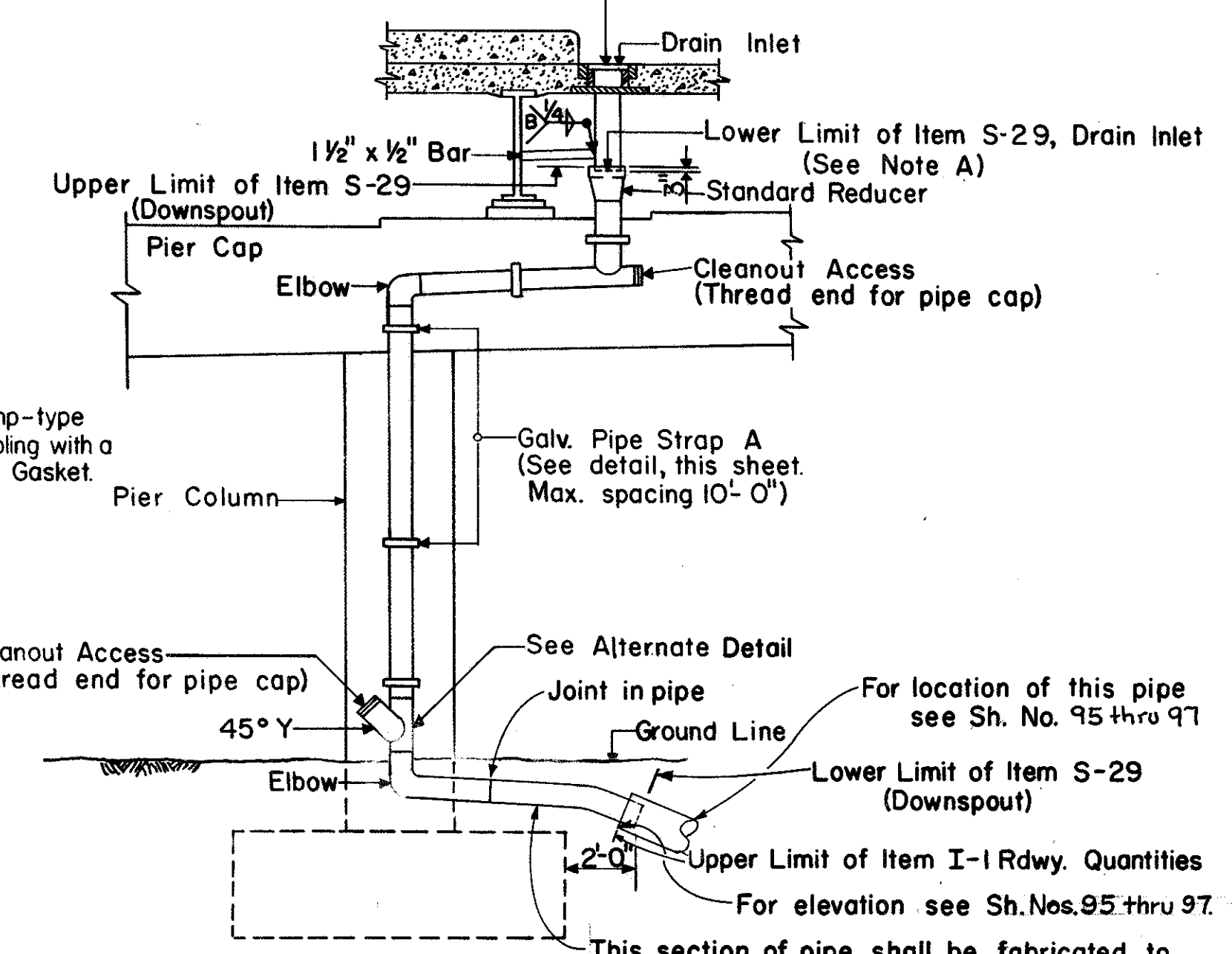
PLAN



PLAN

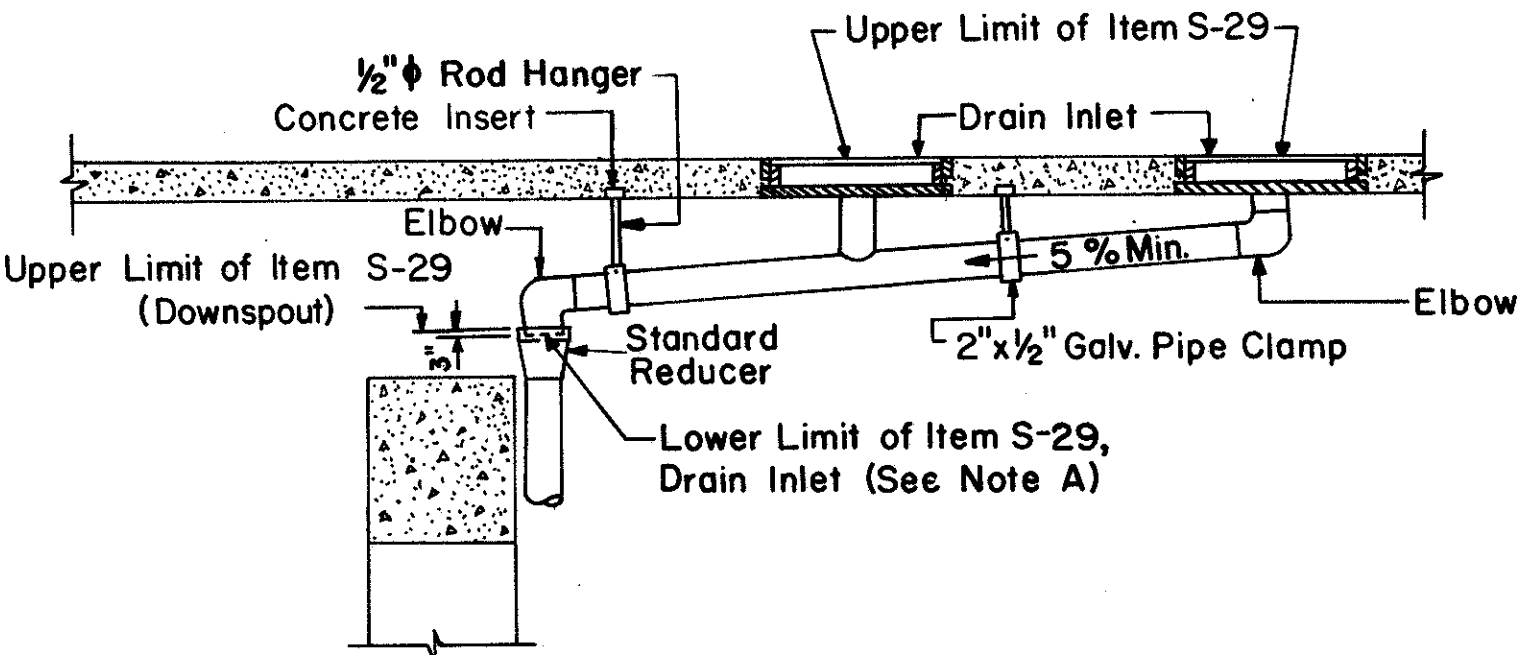
Note: Type of Plan to be used depends upon location of inlet with respect to pier.

For location of drain inlets, see superstructure details.

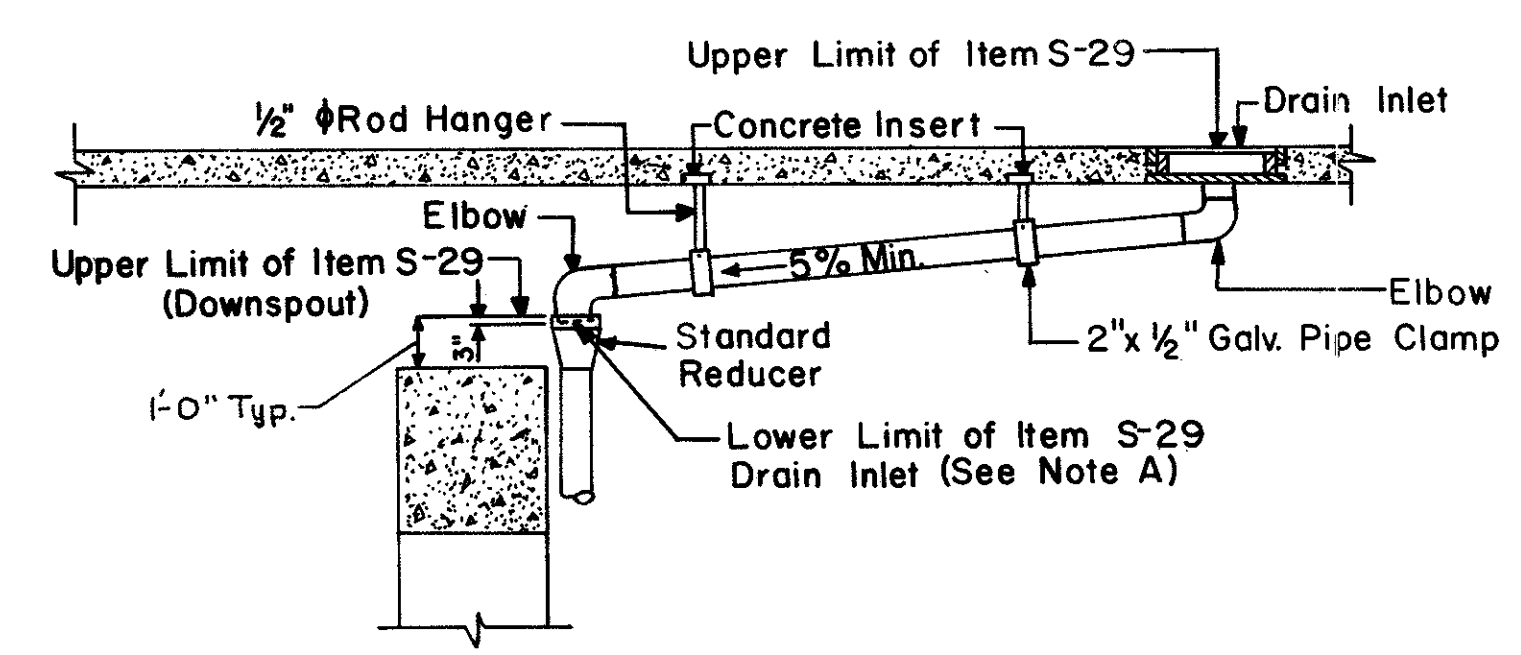


SECTION A-A

NOTE A:
"Drain inlet including Supports and Horizontal Collector System" shall include inlet grating, inlet frame, lugs, concrete insert, 1/2" rod hanger, 2" x 1/2" galvanized pipe clamp, and standard pipe for payment, Item S-29.

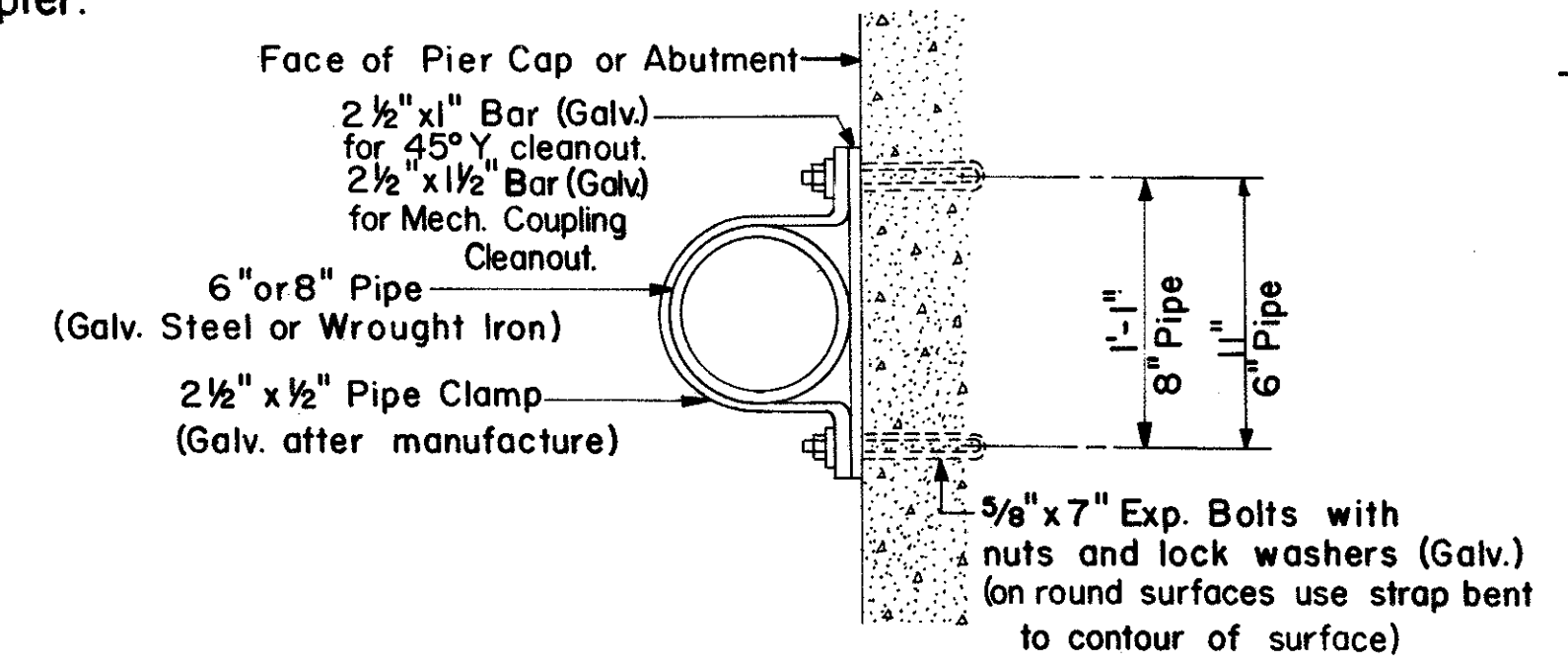


SECTION C-C

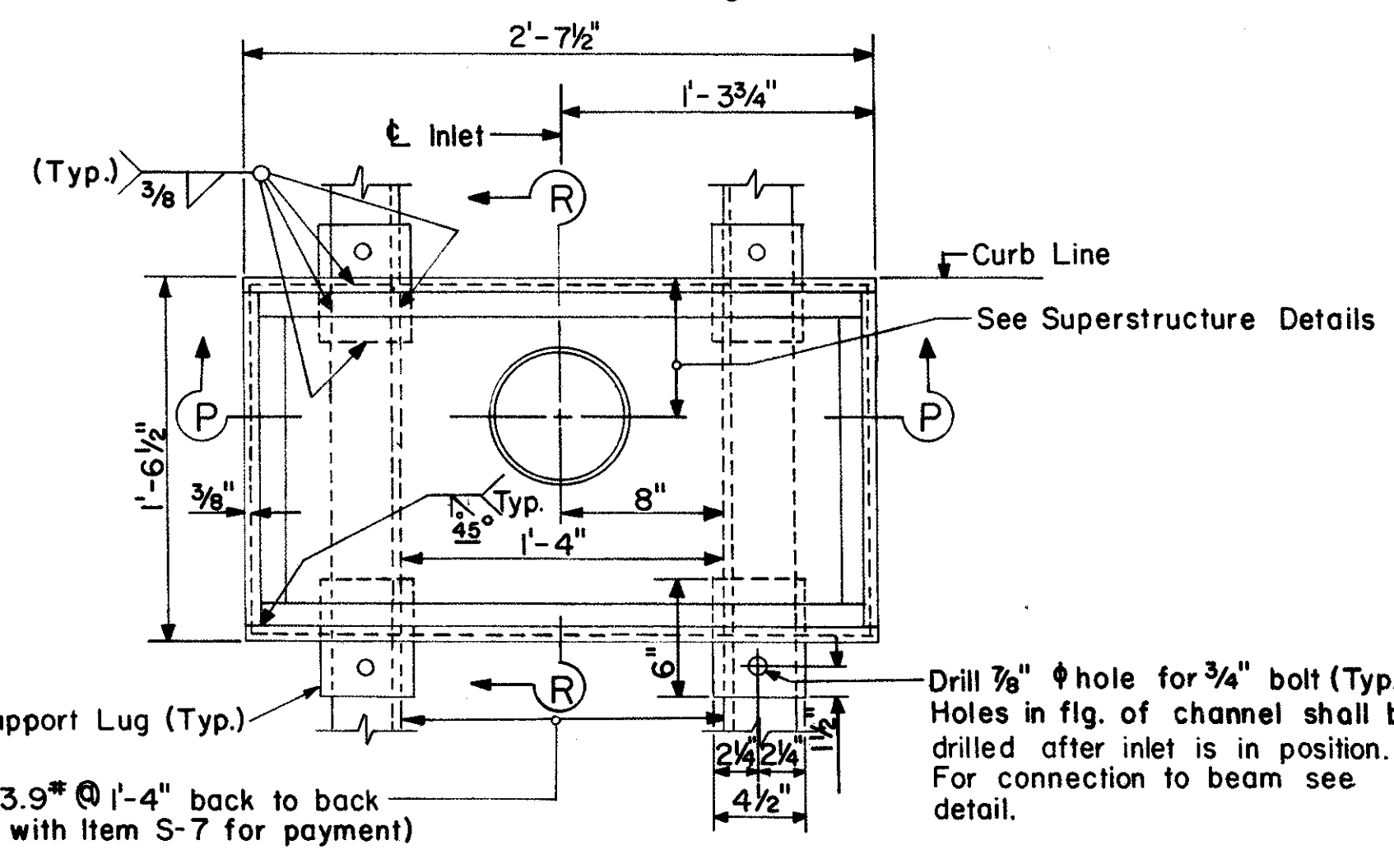


SECTION B-B

NOTE: Downspouts shall be 6" or 8" standard wrought iron pipe or hot-dipped galvanized steel pipe. (For size see superstructure) Joints shall be made by welding or by the use of a clamp type coupling with a ring gasket. All welding shall be done before galvanizing. Straps or clamps for attaching downspouts shall be wrought iron or hot-dipped galvanized steel. On bolts, galvanizing as called for in SEC. M-10.30 will be considered sufficient.



STRAP DETAIL "A"
For mounting on flat surface.

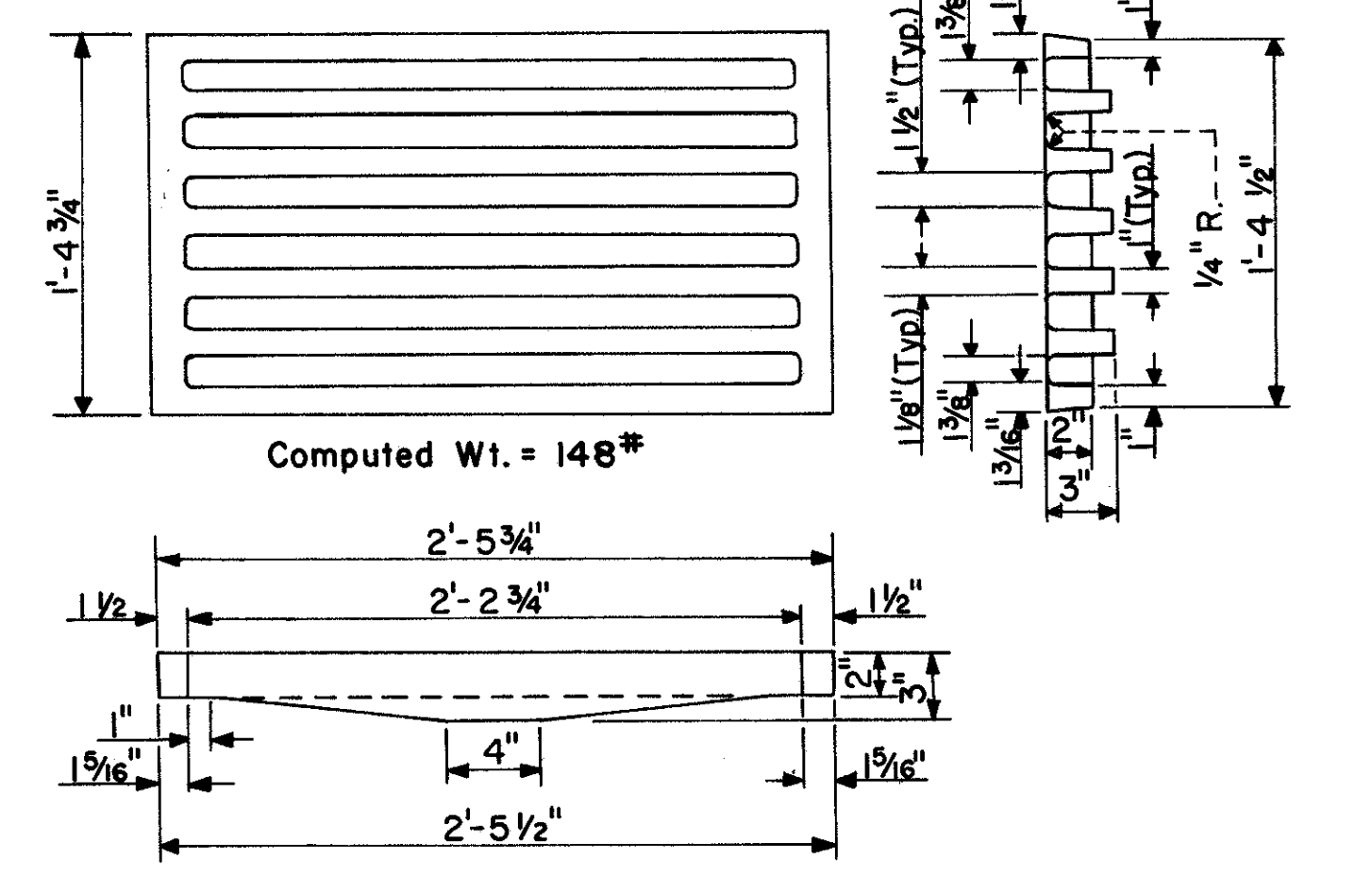


INLET FRAME

NOTES:-
For location of inlets, see superstructure details.

Inlet grating castings shall meet the requirements of Sec. M-7.8 of the Material Specifications of the State of Ohio.
Gratings and Inlets shall be fitted to each other without rattling by grinding grating casting as necessary.

Inlet frame to be welded structural steel plates and standard steel pipe, galvanized after fabrication



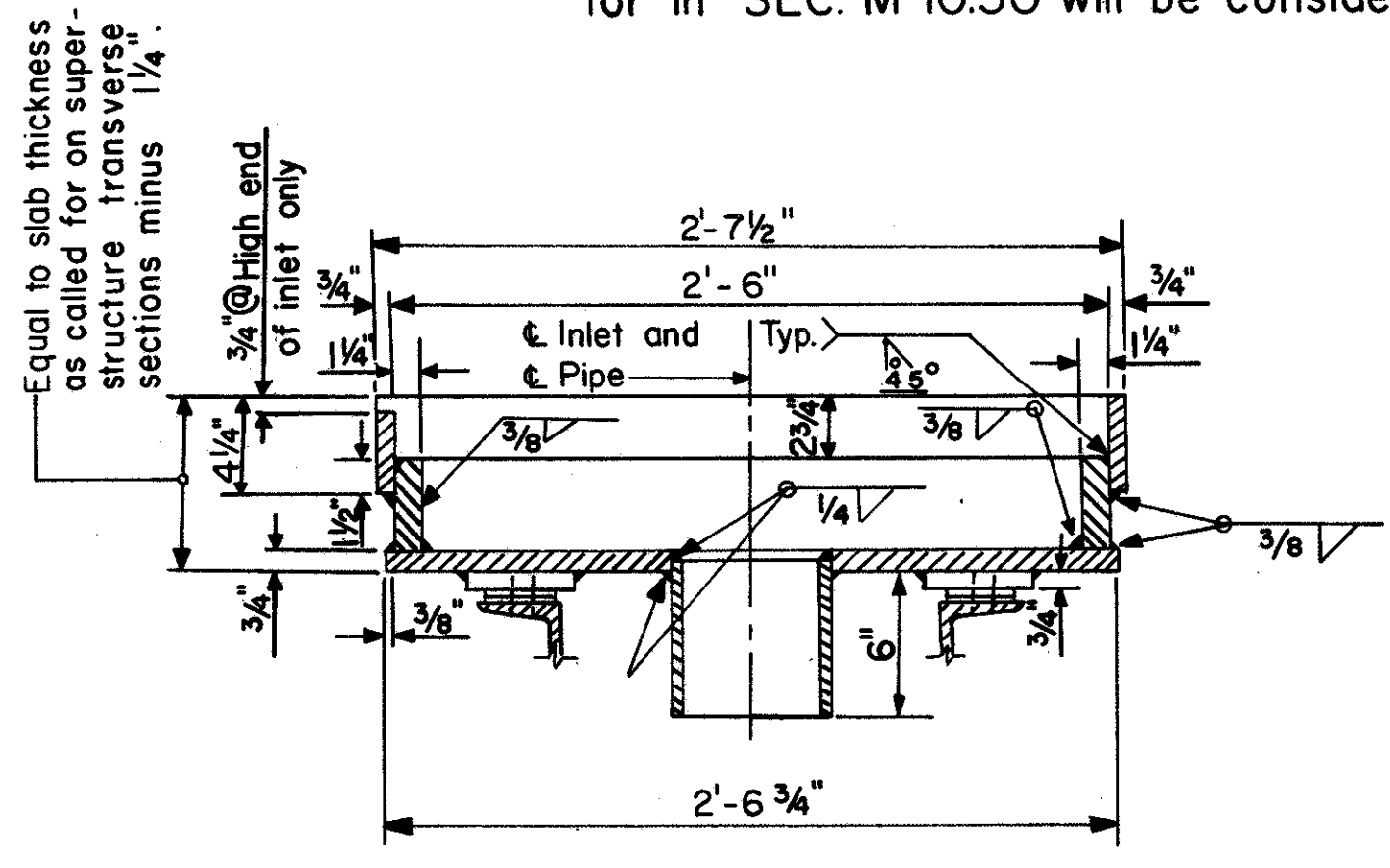
INLET GRATING
(City of Cincinnati Acc. No. 49012)

Weld channels to beam with 5/16" continuous fillet weld. Weld to beam webs only.

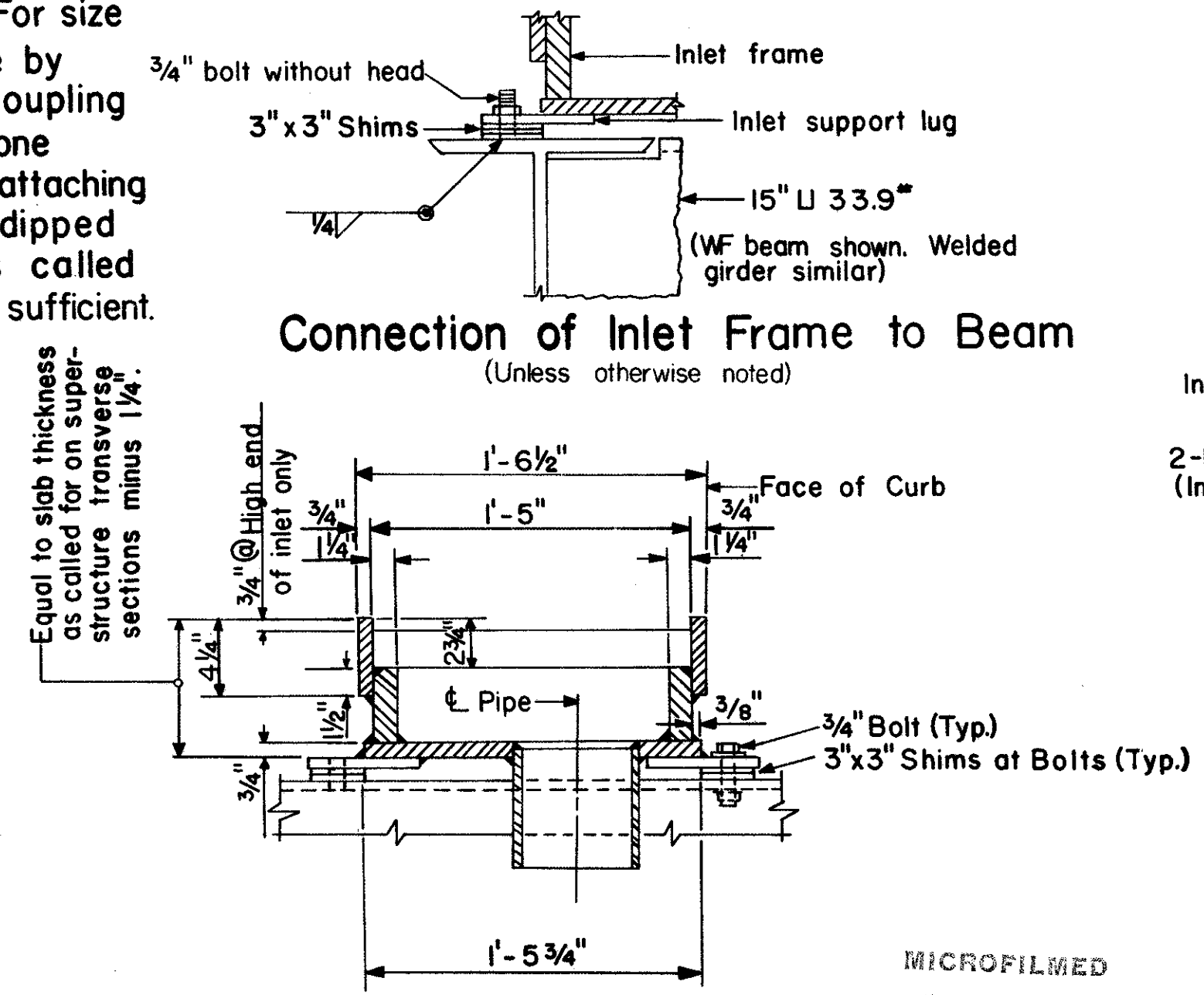
Space lugs on inlets to permit bolting to supporting channels and beams.

Where drain inlet is located adjacent to beams without cover plates, cope channels so top of channel is flush with top of beams.

Where drain inlet is located adjacent to beams with cover plates, cope channels so that the distance top of channel to top of slab is equal to the minimum slab thickness as called for on the superstructure transverse sections.



SECTION P-P

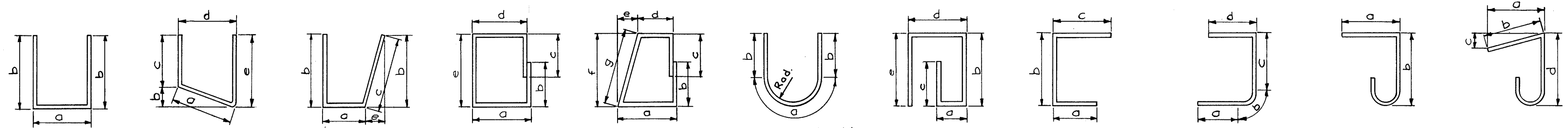


SECTION R-R

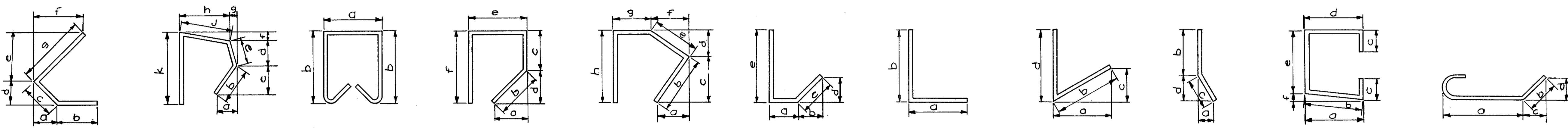
Connection of Inlet Frame to Beam
(Unless otherwise noted)

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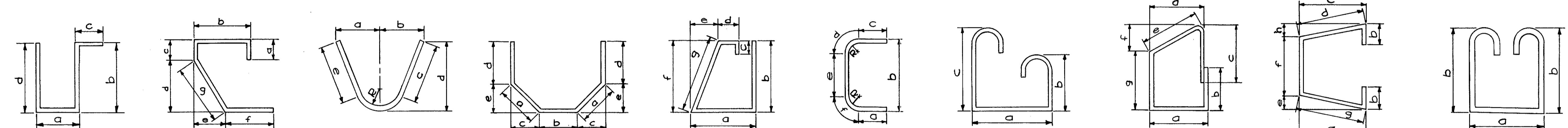
HAZELET & ERDAL CONSULTING ENGINEERS CINCINNATI, OHIO					
TYPICAL DRAINAGE DETAILS					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVISED
		M.J.E.	J.H.O.	2/1/65	
				5-20-65	



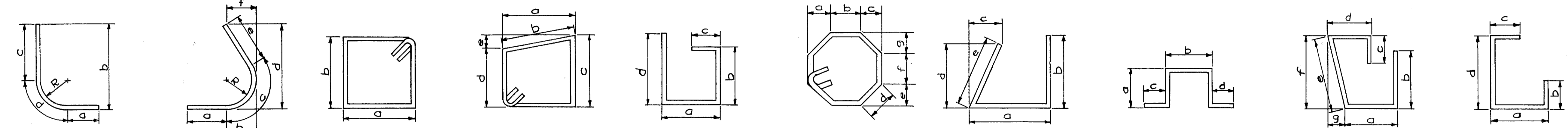
Type 1 Type 2 Type 3 Type 4 Type 5 Type 6 Type 7 Type 8 Type 9 Type 10 Type 11



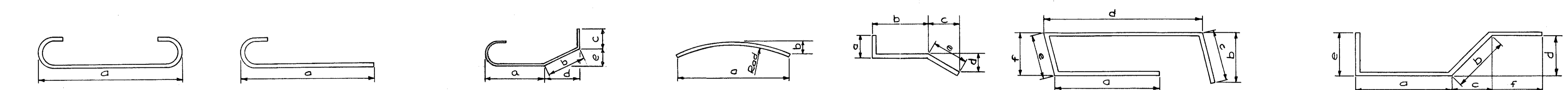
Type 12 Type 13 Type 14 Type 15 Type 16 Type 17 Type 18 Type 19 Type 20 Type 21 Type 22



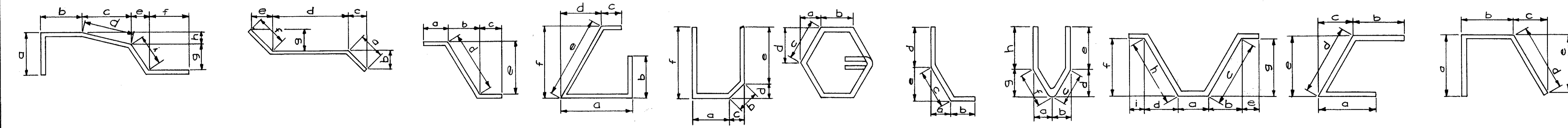
Type 23 Type 24 Type 25 Type 26 Type 27 Type 28 Type 29 Type 30 Type 31 Type 32



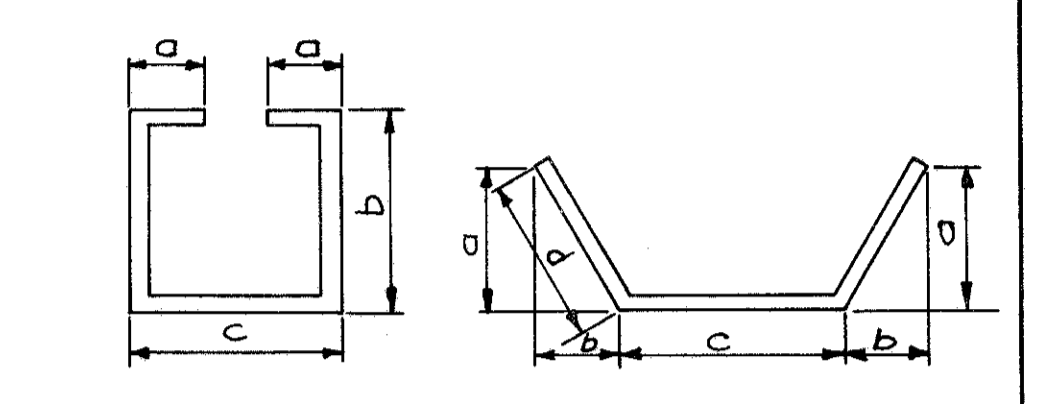
Type 33 Type 34 Type 35 Type 36 Type 37 Type 38 Type 39 Type 40 Type 41 Type 42



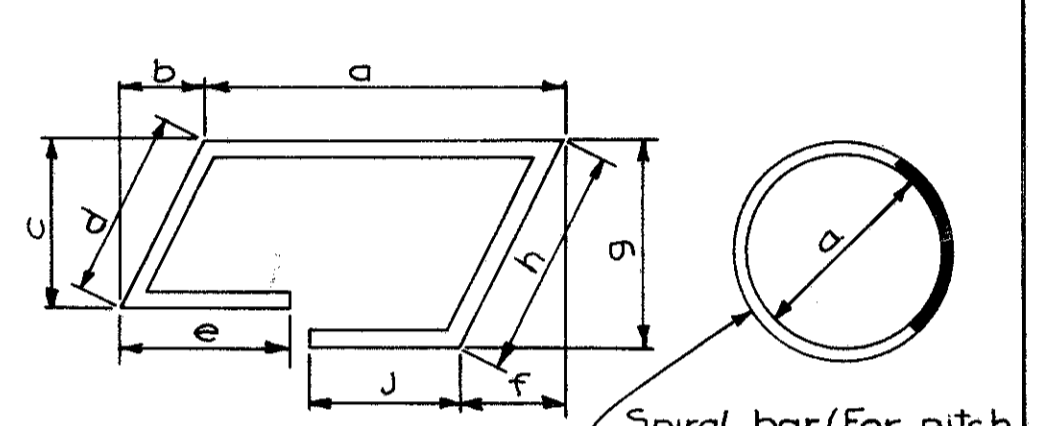
Type 43 Type 44 Type 45 Type 46 Type 47 Type 48 Type 49



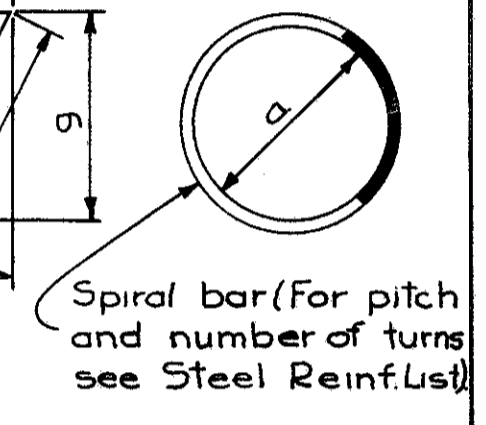
Type 50 Type 51 Type 52 Type 53 Type 54 Type 55 Type 56 Type 57 Type 58 Type 59 Type 60



Type 61 Type 62



Type 63



Spiral bar (For pitch and number of turns see Steel Reinf. List)

Type 64

NOTES

Bars shall be carefully shaped to the pertinent dimensions shown in table I Standard Bends Sheet 212 of the State of Ohio Construction and Material Specifications, unless otherwise indicated above.

Bar size is indicated in the bar mark. The first digit in bar mark beginning with numbers 4 thru 9 indicate the bar size. The first two digits in bar marks beginning with numbers 10 or 11 indicate the bar size.

Spiral reinforcing bars shall not have deformations but shall in other respects conform to Item S-4. 1/2 closed coils shall be provided at the ends of each spiral unit.

Four (or three) steel channel tee or angle spacers, weighing approximately 0.68 lbs. per lin. ft. of spacer, shall be provided for each spiral unit. They shall be equally spaced along the periphery of the coil. The number of pounds of these spacers, based on 0.68 lbs. per lin. ft. will be paid for as reinforcing and is included in the tabulated quantity of spiral bars.

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BAR BENDING SCHEDULE					
DESIGNED	DRAWN	TRACED	CHECKED	REVIEWED DATE	REVIEWED
	M.C.P.		J.H.O. 2/13/65	H.A.S. 5-28-65	

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