

STATE OF OHIO
DEPARTMENT OF TRANSPORTATION

HAS-22-21.26

GREEN TOWNSHIP HARRISON COUNTY

PROJECT DESCRIPTION

IMPROVEMENT OF 400 FEET (0.08 MILES) OF U.S. 22 BY REPLACING THE SUPERSTRUCTURE OF EXISTING BRIDGE HAS-22-2126 OVER WHEELING AND LAKE ERIE RAILROAD, INCLUDING APPROACH PAVEMENT WORK, GUARDRAIL, AND APPROACH SLABS.

EARTH DISTURBED AREA:

PROJECT EARTH DISTURBED AREA:	0.533 ACRES
ESTIMATED CONTRACTOR EARTH DISTURBED AREA:	0.271 ACRES
NOTICE OF INTENT EARTH DISTURBED AREA:	NOT REQUIRED

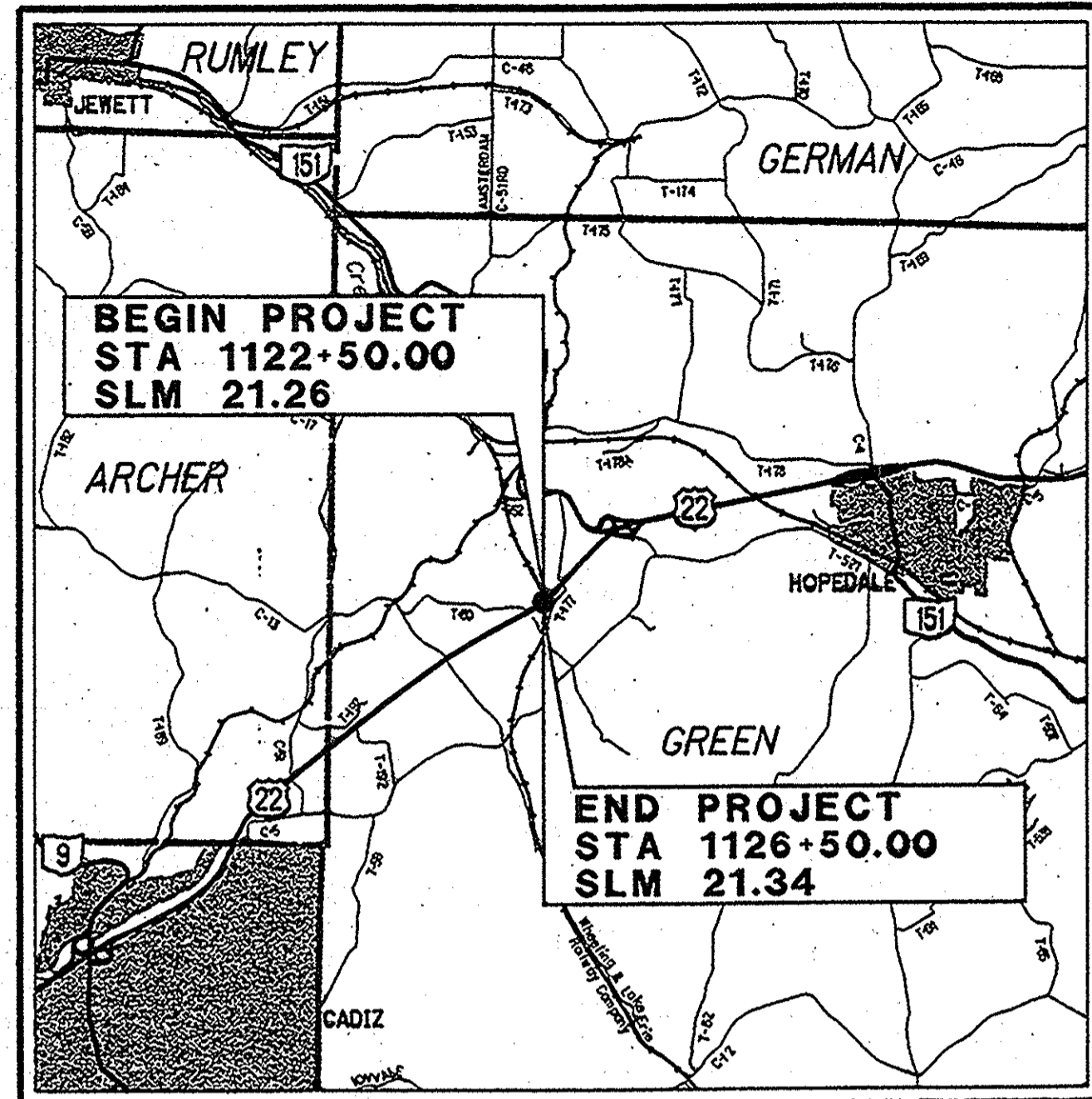
LIMITED ACCESS

THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE OHIO REVISED CODE.

2013 SPECIFICATIONS

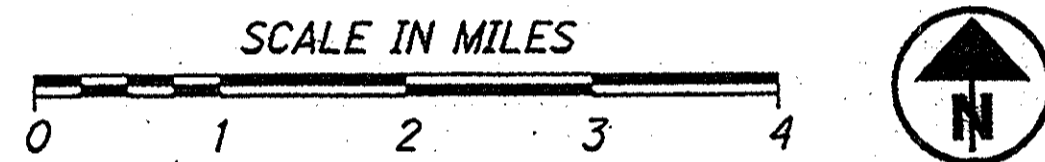
THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, INCLUDING CHANGES AND SUPPLEMENTAL SPECIFICATIONS LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY AND THAT PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE AS SET FORTH ON THE PLANS AND ESTIMATES.



LOCATION MAP

LATITUDE: N 40° 19' 00" LONGITUDE: W 80° 56' 40"



INDEX OF SHEETS:

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DESIGN DESIGNATION

CURRENT ADT (2015)	6200
DESIGN YEAR ADT (2035)	8000
DESIGN HOURLY VOLUME (2035)	800
DIRECTIONAL DISTRIBUTION	53%
TRUCKS (24 HOUR B&C)	21%
DESIGN SPEED	55 MPH
LEGAL SPEED	55 MPH
DESIGN FUNCTIONAL CLASSIFICATION:	
RURAL PRINCIPAL ARTERIAL	
NHS PROJECT	YES

DESIGN EXCEPTIONS
NONE REQUIRED

UNDERGROUND UTILITIES
CONTACT BOTH SERVICES
CALL TWO WORKING DAYS
BEFORE YOU DIG

CALL
1-800-362-2764
(TOLL FREE)

OHIO UTILITIES PROTECTION SERVICE
NON-MEMBERS
MUST BE CALLED DIRECTLY

OIL & GAS PRODUCERS PROTECTIVE
SERVICE CALL: **1-800-925-0988**

PLAN PREPARED BY:
O.D.O.T. DISTRICT 11
PLANNING AND ENGINEERING
NEW PHILADELPHIA, OHIO

ENGINEER'S SEAL:

SIGNED: *Raymond Paul Trivoli*
DATE: DEC 3, 2013

STANDARD CONSTRUCTION DRAWINGS						SUPPLEMENTAL SPECIFICATIONS			
BP-3.1	4/20/12	DM-1.1	1/18/13	MT-96.11	7/19/13	AS-1-81	1/18/13	800	1/17/2014
BP-5.1	7/19/13	DM-4.1	7/19/13	MT-96.20	7/19/13	GSD-1-96	7/19/02	832	10/18/2013
		DM-4.2	7/20/12	MT-96.26	7/19/13	PCB-91	1/18/13		
F-2.1	7/19/13	DM-4.3	7/19/13			SBR-1-99	7/19/02		
F-3.1	7/19/13	DM-4.4	7/20/12	MT-97.10	7/19/13	SICD-1-96	7/19/02		
F-3.3	7/19/13			MT-101.70	7/19/13				
F-3.4	7/19/13	TC-41.20	10/18/13	MT-105.10	7/19/13				
		TC-42.20	10/18/13						
MGS-1.1	7/19/13	TC-52.10	10/18/13						
MGS-2.1	7/19/13	TC-52.20	1/18/13						
MGS-3.1	7/19/13	TC-61.30	4/20/12						
MGS-4.3	1/18/13	TC-65.10	4/20/12						
		TC-65.11	4/20/12						
RM-4.2	10/15/10	TC-73.10	4/20/12						

SPECIAL PROVISIONS

APPROVED *Abdul V. MacLellan, P.E., P.S.*
DATE 12/04/13 DISTRICT DEPUTY DIRECTOR

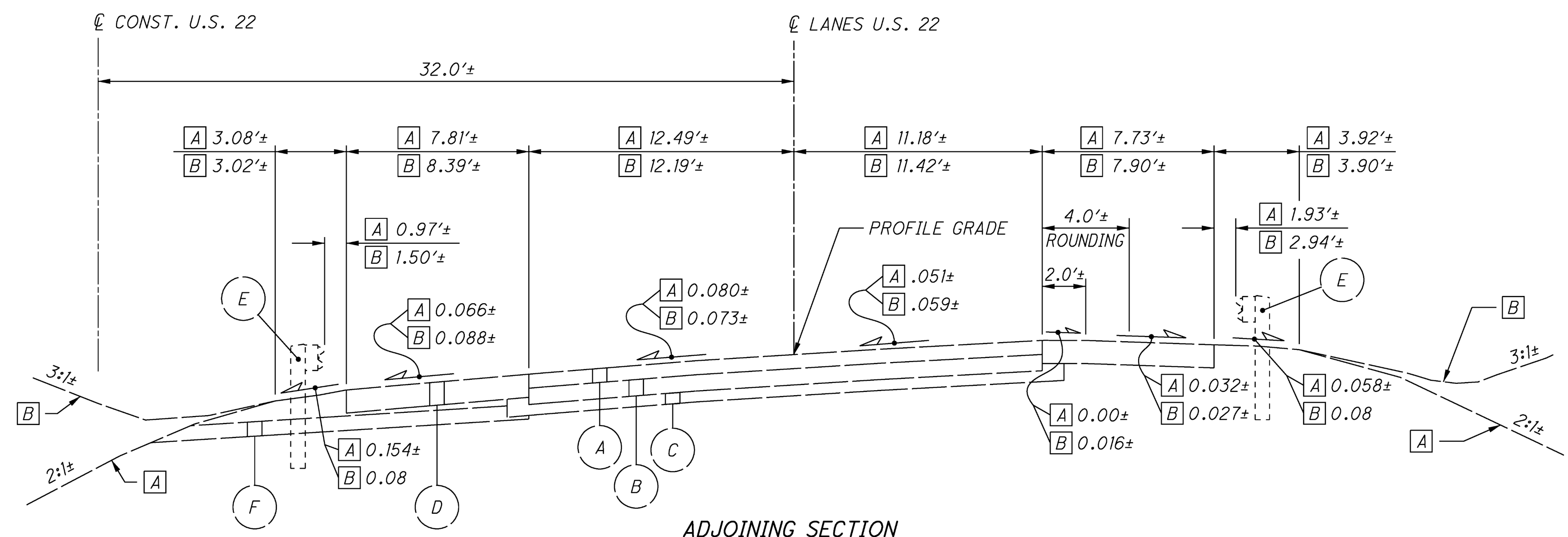
APPROVED *George Whaley*
DATE 12-20-13 DIRECTOR, DEPARTMENT OF
TRANSPORTATION

HAS - US 22-21.260
140137 PID - 88904
Dist 11 3/13/2014

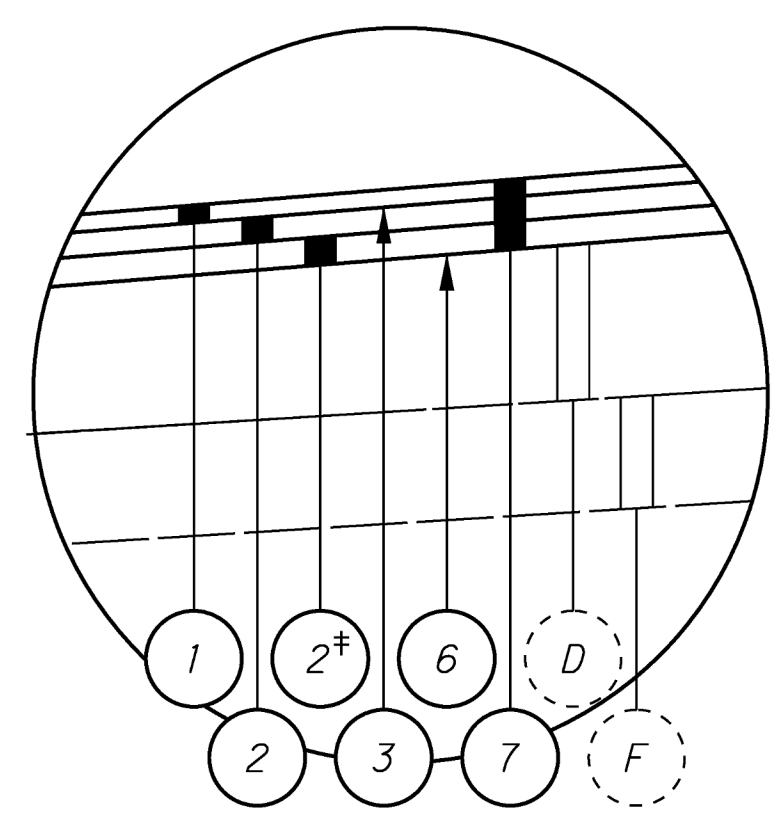
Contract Proposal Available
@ www.contracts.dot.
state.oh.us/home

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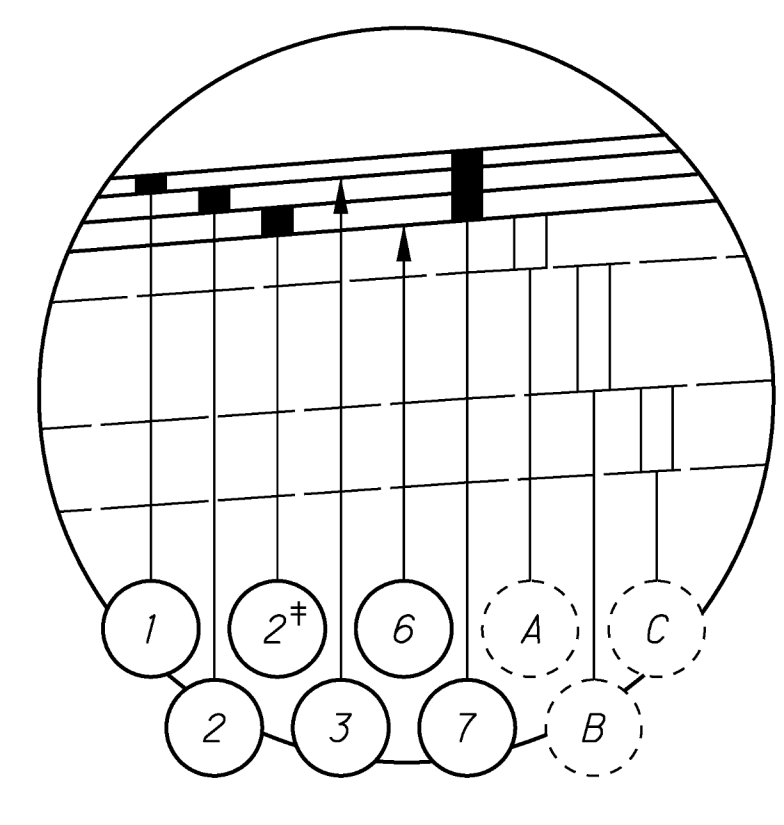
FEDERAL PROJECT NO. **E130 (091)**
PID NO. **88904**
CONSTRUCTION PROJECT NO. **WHEELING & LAKE ERIE**
HAS-22-21.26
1/52



ADJOINING SECTION
 A— BEGIN PROJECT STA. 1122+50.00
 B— END PROJECT STA. 1126+50.00



SHOULDER DETAIL



PLANING DETAIL

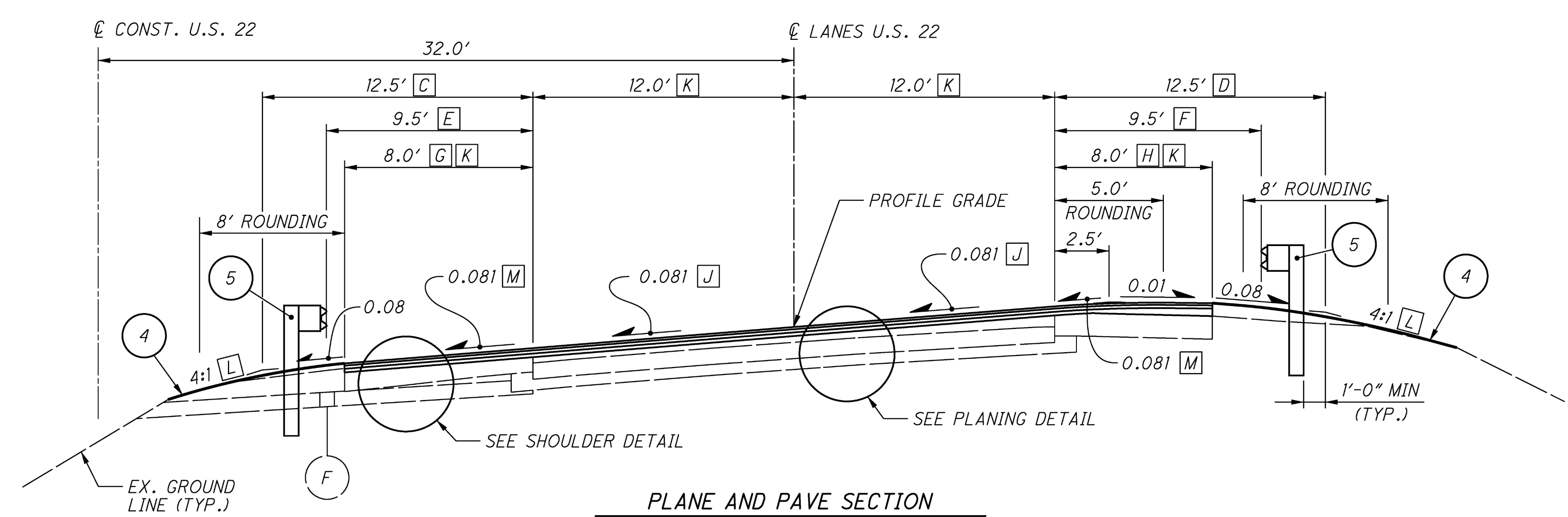
EXISTING LEGEND

- A — 7¾"± EXISTING ASPHALT CONCRETE PAVEMENT
- B — 9"± EXISTING REINFORCED CONCRETE
- C — 6"± EXISTING SUBBASE
- D — 12¾"± EXISTING ASPHALT BASE SHOULDER
- E — EXISTING GUARDRAIL
- F — EXISTING AGGREGATE UNDERDRAINS

LEGEND

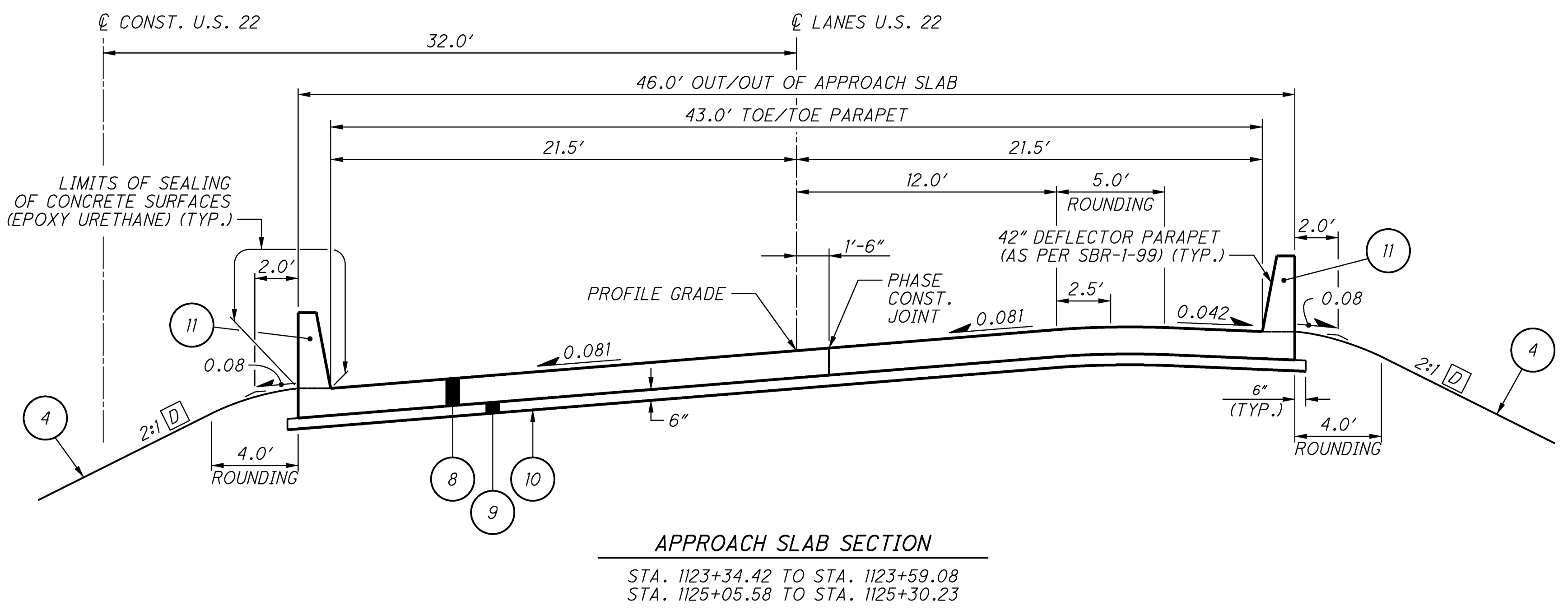
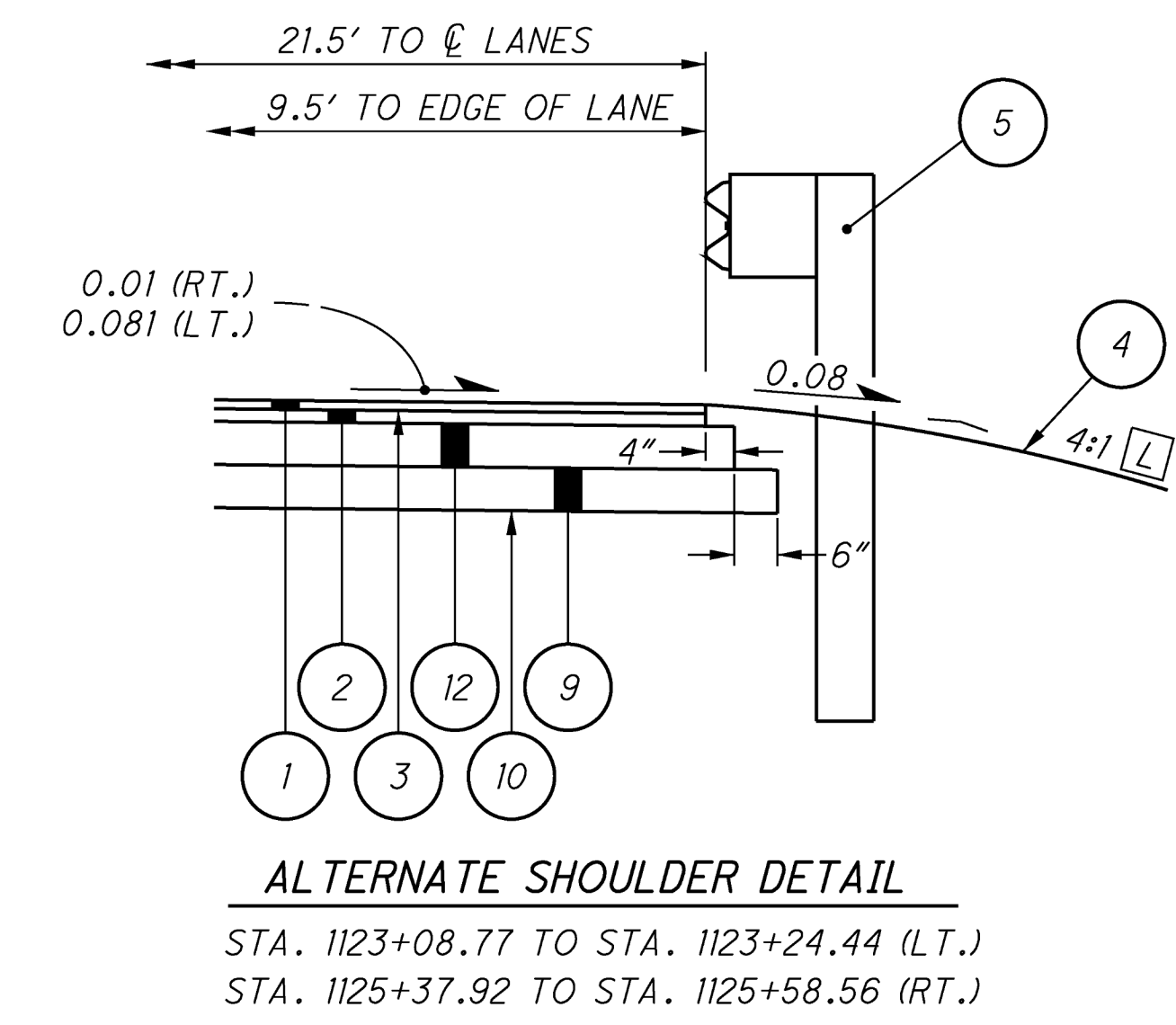
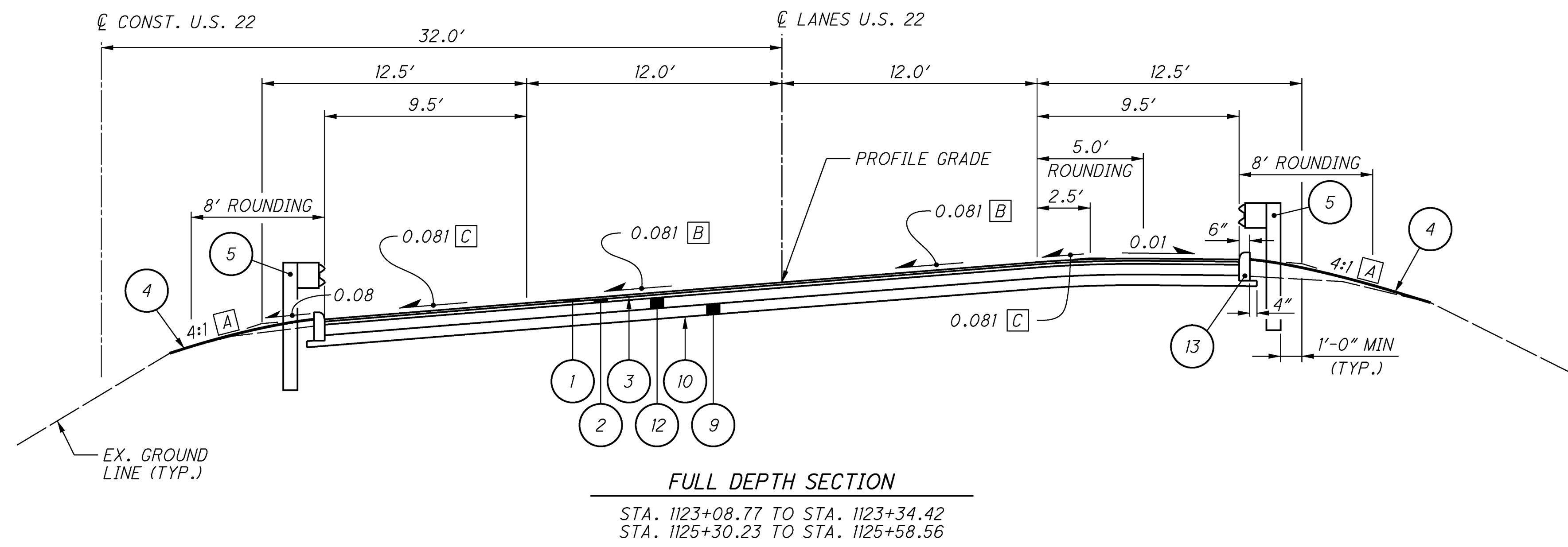
- 1 — ITEM 448 - 1¼" ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22, AS PER PLAN
- 2 — ITEM 448 - 1¾" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22
- 2* — ITEM 448 - ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG64-22 (VARIABLE DEPTH 0"-4½").
- 3 — ITEM 407 - TACK COAT FOR INTERMEDIATE COURSE (APPLIED @ 0.04 GAL./S.Y.)
- 4 — ITEM 659 - SEEDING AND MULCHING
- 5 — ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
- 6 — ITEM 407 - TACK COAT (APPLIED @ 0.075 GAL./S.Y.)
- 7 — ITEM 254 - PAVEMENT PLANING, ASPHALT CONCRETE (VARIABLE DEPTH)
- 8 — ITEM 526 - REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN
- 9 — ITEM 304 - 6" AGGREGATE BASE
- 10 — ITEM 204 - SUBGRADE COMPACTION
- 11 — ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET)
- 12 — ITEM 301 - 6" ASPHALT CONCRETE BASE, PG64-22
- 13 — ITEM 609 - CURB, TYPE 4-C

- C VARIES FROM 10.89'± AT STA. 1122+50.00 TO 12.5' AT STA 1122+75.00
 VARIES FROM 12.5' AT STA. 1126+25.00 TO 11.41'± AT STA 1126+50.00
- D VARIES FROM 11.65'± AT STA. 1122+50.00 TO 12.5' AT STA 1122+75.00
 VARIES FROM 12.5' AT STA. 1126+25.00 TO 11.80'± AT STA 1126+50.00
- E VARIES FROM 8.78'± AT STA. 1122+50.00 TO 9.5' AT STA 1122+75.00
 VARIES FROM 9.5' AT STA. 1126+25.00 TO 9.89'± AT STA 1126+50.00
- F VARIES FROM 9.66'± AT STA. 1122+50.00 TO 9.5' AT STA 1122+75.00
 VARIES FROM 9.5' AT STA. 1126+25.00 TO 10.84'± AT STA 1126+50.00
- G 8.67' FROM STA. 1123+00.00 TO STA. 1123+08.51
- H 7.25' FROM STA. 1123+00.00 TO STA. 1123+08.51
- J SEE SUPERELEVATION TABLE ON SHEET NO. 22.
- K FOR PAVEMENT AND SHOULDER TRANSITIONS TO MATCH EXISTING, SEE PLAN AND PROFILE SHEETS
- L OR AS SHOWN ON CROSS SECTIONS
- M OR MATCH SUPERELEVATION



PLANE AND PAVE SECTION
 STA. 1122+50.00 TO STA. 1123+08.51
 STA. 1125+58.81 TO STA. 1126+50.00

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- [A] OR AS SHOWN ON CROSS SECTIONS
- [B] SEE SUPERELEVATION TABLE ON SHEET NO. 22.
- [C] OR MATCH SUPERELEVATION
- [D] VARIES FROM 2:1 AT STA. 1125+05.58 TO 4:1 AT STA. 1125+30.23

NOTES

1. TRANSITION FROM THE TYPICAL SECTION TO THE BRIDGE DECK TRANSVERSE SECTION OVER THE LENGTH OF THE APPROACH SLABS. THE APPROACH SLAB SECTION SHOWN DEPICTS THE CONFIGURATION NEAREST THE BRIDGE LIMITS.
2. FOR LEGEND SEE SHEET 2.

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UTILITIES

THERE ARE NO KNOWN UNDERGROUND OR OVERHEAD UTILITIES WITHIN THE PROJECT CONSTRUCTION LIMITS.

ROUNDING

THE ROUNDING AT SLOPE BREAKPOINTS SHOWN ON THE TYPICAL SECTIONS APPLIES TO ALL CROSS-SECTIONS EVEN THOUGH OTHERWISE SHOWN.

EXISTING PLANS

THE FOLLOWING EXISTING PLANS ARE AVAILABLE FOR REFERENCE AT THE DISTRICT II OFFICE OF THE OHIO DEPARTMENT OF TRANSPORTATION, 2201 REISER AVE., S.E., NEW PHILADELPHIA, OHIO 44663:

ORIGINAL CONSTRUCTION:
HAS-22-18.97 / JEF-22-0.00 (1960)

RESURFACING AND DECK OVERLAY:
HAS-22-20.07 (1980)
HAS-22-15.03 (1990)

RESURFACING:
HAS-22-30.674, PID: 18184 (1999)
HAS-22-11.75, PID: 81739 (2011)

ADDITIONALLY THE EXISTING PLANS CAN BE FOUND ON THE DEPARTMENTS WEBSITE AT THE FOLLOWING ADDRESS:

<http://www.dot.state.oh.us/Divisions/ContractAdmin/Contracts/Pages/designfiles.aspx>

SURVEYING PARAMETERS

USE THE FOLLOWING VERTICAL POSITIONING AND HORIZONTAL POSITIONING PARAMETERS FOR ALL SURVEYING:

VERTICAL POSITIONING ASSUMED

HORIZONTAL POSITIONING

REFERENCE FRAME: NAD83 (CORS96)
ELLIPSOID: GRS 1980
MAP PROJECTION: LAMBERT CONIC
COORDINATE SYSTEM: OHIO STATE PLANE NORTH ZONE
COMBINED SCALE FACTOR: 0.9999746434

UNITS ARE IN U.S. SURVEY FEET.

WORK LIMITS

THE WORK LIMITS SHOWN ON THESE PLANS ARE FOR PHYSICAL CONSTRUCTION ONLY. PROVIDE THE INSTALLATION AND OPERATION OF ALL WORK ZONE TRAFFIC CONTROL AND WORK ZONE TRAFFIC CONTROL DEVICES REQUIRED BY THESE PLANS WHETHER INSIDE OR OUTSIDE THESE WORK LIMITS.

CLEARING AND GRUBBING

ALTHOUGH THERE ARE NO TREES OR STUMPS SPECIFICALLY MARKED FOR REMOVAL WITHIN THE LIMITS OF THE PROJECT, A LUMP SUM QUANTITY IS INCLUDED IN THE GENERAL SUMMARY FOR ITEM 201, CLEARING AND GRUBBING. ALL PROVISIONS AS SET FORTH IN THE SPECIFICATIONS UNDER THIS ITEM ARE INCLUDED IN THE LUMP SUM PRICE BID FOR ITEM 201, CLEARING AND GRUBBING.

CONNECTION BETWEEN EXISTING AND PROPOSED GUARDRAIL

WHEN IT IS NECESSARY TO SPLICE PROPOSED GUARDRAIL TO EXISTING GUARDRAIL, ONLY THE EXISTING GUARDRAIL SHALL BE CUT, DRILLED, OR PUNCHED. THE CONNECTION SHALL BE MADE USING A W-BEAM, BEAM SPLICE AS SHOWN IN AASHTO M 180-12, EXCEPT THE BEAM WASHERS ARE NOT TO BE USED. PAYMENT SHALL BE INCLUDED IN THE CONTRACT PRICE FOR THE RESPECTIVE GUARDRAIL ITEMS.

ITEM 606 - ANCHOR ASSEMBLY, MGS TYPE E

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING ANY OF THE GUARDRAIL END TERMINALS FOR THE MGS GUARDRAIL AS LISTED ON ROADWAY ENGINEERING'S WEB PAGE UNDER ROADSIDE SAFETY DEVICES FOR APPROVED GUARDRAIL END TREATMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

THE FACE OF THE TYPE E IMPACT HEAD SHALL BE COVERED WITH A SHEET OF TYPE G REFLECTIVE SHEETING, PER CMS 730.19.

REFER TO THE MANUFACTURERS INSTRUCTIONS REGARDING THE INSTALLATION OF, AND THE GRADING AROUND THE FOUNDATION TUBES AND GROUND STRUT. THE TOP OF ANY FOUNDATION TUBE SHOULD BE LESS THAN 4 INCHES ABOVE THE GROUND. THE PLACEMENT OF THE FOUNDATION TUBES SHOULD BE AN APPROPRIATE DEPTH BELOW THE LEVEL LINE IN ORDER TO MAINTAIN THE FINISHED GUARDRAIL HEIGHT OF 27.75 INCHES FROM THE EDGE OF THE SHOULDER.

ON-SITE GRADING IS REQUIRED IF THE TOP OF THE FOUNDATION TUBES OR TOP OF THE GROUND STRUT DOES PROJECT MORE THAN 4 INCHES ABOVE THE GROUND LINE.

PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 606, ANCHOR ASSEMBLY, MGS TYPE E, EACH, AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT A COMPLETE AND FUNCTIONAL ANCHOR ASSEMBLY SYSTEM, INCLUDING ALL RELATED TRANSITIONS, REFLECTIVE SHEETING, HARDWARE, GRADING, EMBANKMENT AND EXCAVATION NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

ITEM 448 - ASPHALT CONCRETE SURFACE COURSE, TYPE 1, PG64-22, AS PER PLAN

FOLLOW SPECIFICATION 703.05 EXCEPT DO NOT USE COARSE AGGREGATE FROM A SOURCE DESIGNATED 'SR' OR 'SRH' ACCORDING TO THE OFFICE OF MATERIALS MANAGEMENT (OMM) IN ANY JOB MIX FORMULA (JMF) FOR THIS ITEM.

PART-WIDTH CONSTRUCTION

BECAUSE OF THE NECESSITY TO BUILD THIS PROJECT UNDER TRAFFIC AND TO CONSTRUCT THE FULL PAVEMENT WIDTH IN STAGES, EXERCISE CARE TO PREVENT THE CONSTRUCTION OF A BUTT JOINT IN THE BASE COURSES. LAP LONGITUDINAL JOINTS AS SHOWN ON STANDARD CONSTRUCTION DRAWING BP-3.1.

FENCE LENGTHS

THE LENGTHS OF FENCE SHOWN IN THE PLANS ARE HORIZONTAL DIMENSIONS. MEASUREMENTS OF THE FINAL QUANTITIES WILL BE IN ACCORDANCE WITH ITEM 607.

SEEDING AND MULCHING

SEEDING AND MULCHING SHALL BE APPLIED TO ALL AREAS OF EXPOSED SOIL BETWEEN THE RIGHT-OF-WAY LINES, AND WITHIN THE CONSTRUCTION LIMITS FOR AREAS OUTSIDE THE RIGHT-OF-WAY LINES COVERED BY WORK AGREEMENT OR SLOPE EASEMENT. QUANTITY CALCULATIONS FOR SEEDING AND MULCHING ARE BASED ON THESE LIMITS.

WETLAND AVOIDANCE

WETLAND AREAS HAVE BEEN IDENTIFIED IN THE PROJECT VACINITY. THESE AREAS ARE SHOWN ON SHEETS 13 AND 24 OF THE PLANS. THESE WETLAND AREAS ARE NOT WITHIN THE PROJECT'S CONSTRUCTION LIMITS. HOWEVER, THE CONTRACTOR SHALL EXERCISE CAUTION TO ASSURE THAT NO IMPACTS OCCUR TO THESE WETLAND AREAS.

HORIZONTAL CONTROL POINTS					
STATION	OFFSET	NORTH	EAST	ELEV.	REMARKS
1122+88.19	℄	240,664.579	2,402,310.699	- - -	I. PIN SET
1123+25.88	58.17' RT.	240,637.478	2,402,374.771	1152.76	BENCH MARK #1
1126+40.57	6.53' RT.	240,875.170	2,402,593.489	1153.26	BENCH MARK #2
1126+11.52	℄	240,860.667	2,402,567.444	- - -	I. PIN SET

ITEM 614, MAINTAINING TRAFFIC
 THE CONTRACTOR SHALL MAINTAIN TRAFFIC AT ALL TIMES AND IN ACCORDANCE WITH THE REQUIREMENTS OF ITEM 614 AND THE PHASE CONSTRUCTION DETAILS DESCRIBED ON SHEET 27. TRAFFIC SHALL BE MAINTAINED AT ALL TIMES BY USE OF THE EXISTING PAVEMENT AND PORTIONS OF THE EXISTING AND PROPOSED BRIDGE.

LENGTH AND DURATION OF LANE CLOSURES AND RESTRICTIONS SHALL BE AT THE APPROVAL OF THE ENGINEER. IT IS THE INTENT TO MINIMIZE THE IMPACT TO THE TRAVELING PUBLIC. LANE CLOSURES OR RESTRICTIONS OVER SEGMENTS OF THE PROJECT IN WHICH NO WORK IS ANTICIPATED WITHIN A REASONABLE TIME FRAME, AS DETERMINED BY THE ENGINEER, SHALL NOT BE PERMITTED. THE LEVEL OF UTILIZATION OF MAINTENANCE OF TRAFFIC DEVICES SHALL BE COMMENSURATE WITH THE WORK IN PROGRESS.

ALTERNATING ONE-WAY TRAFFIC SHALL BE MAINTAINED DURING PHASE 1 THROUGH PHASE 2 BY USE OF WORK ZONE TRAFFIC SIGNALS AS SHOWN ON SHEETS 6-8. TRAFFIC SHALL BE SEPARATED FROM THE WORK AREA BY MEANS OF ITEM 622, PORTABLE BARRIER, 32".

PRIOR TO THE BEGINNING OF ANY CONSTRUCTION THAT WILL REQUIRE THE CLOSURE OF EXISTING LANES TO TRAFFIC, ALL WORK ZONE SIGNALS, SIGNS, LIGHTS, PORTABLE CONCRETE BARRIER, AND WORK ZONE PAVEMENT MARKINGS SHALL BE FURNISHED AND INSTALLED AS SHOWN ON SHEETS 6-8. WORK ZONE PAVEMENT MARKINGS AND PORTABLE BARRIER INSTALLATION SHALL BE ACCOMPLISHED IN ONE DAY, WITH FLAGGERS BEING UTILIZED FOR THE PROTECTION OF TRAFFIC DURING THE INSTALLATION OF THESE ITEMS. WHEN THE ABOVE REQUIREMENTS HAVE BEEN SATISFIED, SIGNAL CONTROLLED ALTERNATING ONE-WAY TRAFFIC MAY BEGIN.

ALL WORK AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH CMS 614 AND OTHER APPLICABLE PORTIONS OF THE SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC, UNLESS SEPARATELY ITEMIZED IN THE PLAN.

ITEM 614, BARRIER REFLECTORS AND OBJECT MARKERS
 BARRIER REFLECTORS AND OBJECT MARKERS SHALL BE INSTALLED ON ALL PORTABLE BARRIER (PB) USED FOR TRAFFIC CONTROL. BARRIER REFLECTORS SHALL BE INSTALLED ON ALL GUARDRAIL AND EXISTING BRIDGE RAILING USED FOR TRAFFIC CONTROL. BARRIER REFLECTORS, OBJECT MARKERS AND THEIR INSTALLATION SHALL CONFORM TO CMS 626, EXCEPT THAT THE SPACING SHALL BE 50 FEET.

IN ADDITION TO QUANTITIES ON SHEET NO. 6, THE FOLLOWING QUANTITY HAS BEEN CARRIED TO THE GENERAL SUMMARY FOR USE DURING PHASE 1 AND IS TO BE APPLIED TO THE EXISTING LEFT SIDE GUARDRAIL AND BRIDGE RAILING.

ITEM 614, BARRIER REFLECTOR, TYPE A2 21 EACH

NOTIFICATION OF WORK ZONE LANE RESTRICTIONS
 THE CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST EIGHTEEN (18) DAYS PRIOR TO IMPLEMENTING ANY WORK ZONE RESTRICTIONS THAT WILL REDUCE THE WIDTH OR VERTICAL CLEARANCE OF ANY LANE ON WHICH TRAFFIC WILL BE MAINTAINED DURING CONSTRUCTION.

FLOODLIGHTING
 FLOODLIGHTING OF THE WORK SITE FOR OPERATIONS CONDUCTED DURING NIGHTTIME PERIODS SHALL BE ACCOMPLISHED SO THAT THE LIGHTS DO NOT CAUSE GLARE TO THE DRIVERS ON THE ROADWAY. TO ENSURE THE ADEQUACY OF THE FLOODLIGHT PLACEMENT, THE CONTRACTOR AND THE ENGINEER SHALL DRIVE THROUGH THE WORK SITE EACH NIGHT WHEN THE LIGHTING IS IN PLACE AND OPERATIVE PRIOR TO COMMENCING ANY WORK. IF GLARE IS DETECTED, THE LIGHT PLACEMENT AND SHIELDING SHALL BE ADJUSTED TO THE SATISFACTION OF THE ENGINEER BEFORE WORK PROCEEDS.

PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC.

ITEM 614 - WORK ZONE IMPACT ATTENUATOR FOR 24" WIDE HAZARDS (BIDIRECTIONAL)

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING A NON-GATING IMPACT ATTENUATOR. FURNISH AN IMPACT ATTENUATOR FROM THE OFFICE OF ROADWAY ENGINEERING APPROVED LIST FOR WORK ZONE IMPACT ATTENUATORS. THE APPROVED LIST IS AVAILABLE AT THE "ROADWAY STANDARDS: PROPRIETARY ROADSIDE SAFETY DEVICES" WEB PAGE ON THE OFFICE OF ROADWAY ENGINEERING WEBSITE.

INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

THE CONTRACTOR SHALL REPAIR OR REPLACE A DAMAGED UNIT WITHIN 24 HOURS OF A DAMAGING IMPACT.

WHEN BIDIRECTIONAL DESIGNS ARE SPECIFIED, THE CONTRACTOR SHALL SUPPLY APPROPRIATE TRANSITIONS.

WHEN GATING IMPACT ATTENUATORS ARE DESIRED, THE CONTRACTOR SHALL SUBMIT DOCUMENTATION TO THE ENGINEER FOR ACCEPTANCE.

THE COST FOR THE ADDITIONAL BARRIER REQUIRED FOR A GATING IMPACT ATTENUATOR SHALL BE INCLUDED IN THE COST OF THE GATING IMPACT ATTENUATOR.

PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT AND MAINTAIN A COMPLETE AND FUNCTIONAL IMPACT ATTENUATOR SYSTEM, INCLUDING ALL RELATED BACKUPS, TRANSITIONS, LEVELING PADS, HARDWARE AND GRADING, NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

OVERHEAD MOUNTED WORK ZONE SIGNALS
 SIGNALS SHALL BE OVERHEAD MOUNTED IN ACCORDANCE WITH THE DETAILS SHOWN ON SCD-96.20.

DUST CONTROL
 THE CONTRACTOR SHALL FURNISH AND APPLY WATER FOR DUST CONTROL AS DIRECTED BY THE ENGINEER. THE FOLLOWING ESTIMATED QUANTITY HAS BEEN INCLUDED FOR DUST CONTROL PURPOSES:

ITEM 616, WATER 5 M. GAL.

ITEM 614 - LAW ENFORCEMENT OFFICER (WITH PATROL CAR) FOR ASSISTANCE DURING CONSTRUCTION OPERATIONS

USE OF LAW ENFORCEMENT OFFICERS (LEOs) BY CONTRACTORS OTHER THAN THE USES SPECIFIED BELOW WILL NOT BE PERMITTED AT PROJECT COST. LEOs SHOULD NOT BE USED WHERE THE OMTCD INTENDS THAT FLAGGERS BE USED.

IN ADDITION TO THE REQUIREMENTS OF CMS 614 AND THE OMTCD, A UNIFORMED LEO WITH AN OFFICIAL PATROL CAR (CAR WITH TOP-MOUNTED EMERGENCY FLASHING LIGHTS AND COMPLETE MARKINGS OF THE APPROPRIATE LAW ENFORCEMENT AGENCY) SHALL BE PROVIDED FOR THE FOLLOWING TRAFFIC CONTROL TASKS:

FOR LANE CLOSURES: DURING INITIAL SET-UP PERIODS, TEAR DOWN PERIODS, SUBSTANTIAL SHIFTS OF A CLOSURE POINT OR WHEN NEW LANE CLOSURE ARRANGEMENTS ARE INITIATED FOR LONG-TERM LANE CLOSURES/SHIFTS (FOR THE FIRST AND LAST DAY OF MAJOR CHANGES IN TRAFFIC CONTROL SETUP). IN GENERAL, LEOs SHOULD BE POSITIONED AT THE POINT OF LANE RESTRICTION OR ROAD CLOSURE AND TO MANUALLY CONTROL TRAFFIC MOVEMENTS THROUGH INTERSECTIONS IN WORK ZONES.

LEOs SHOULD NOT FORGO THEIR TRAFFIC CONTROL RESPONSIBILITIES TO APPREHEND MOTORISTS FOR ROUTINE TRAFFIC VIOLATIONS. HOWEVER, IF A MOTORIST'S ACTIONS ARE CONSIDERED TO BE RECKLESS, THEN PURSUIT OF THE MOTORIST IS APPROPRIATE.

THE LEOs WORK AT THE DIRECTION OF THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR SECURING THE SERVICES OF THE LEOs WITH THE APPROPRIATE AGENCIES AND COMMUNICATING THE INTENTIONS OF THE PLANS WITH RESPECT TO DUTIES OF THE LEOs. THE ENGINEER SHALL HAVE FINAL CONTROL OVER THE LEOs' DUTIES AND PLACEMENT, AND WILL RESOLVE ANY ISSUES THAT MAY ARISE BETWEEN THE TWO PARTIES.

THE LEO SHALL REPORT IN TO THE CONTRACTOR PRIOR TO THE START OF THE SHIFT, IN ORDER TO RECEIVE INSTRUCTIONS REGARDING SPECIFIC WORK ASSIGNMENTS DURING HIS/HER SHIFT. THE LEO IS EXPECTED TO STAY AT THE PROJECT SITE FOR THE ENTIRE DURATION OF HIS/HER SHIFT. THE LEO SHALL REPORT TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT. ONCE THE LEO HAS COMPLETED THE DUTIES DESCRIBED ABOVE AND STILL HAS TIME REMAINING ON HIS/HER SHIFT, THE LEO MAY BE ASKED TO PATROL THROUGH THE WORK ZONE (WITH FLASHING LIGHTS OFF) OR BE PLACED AT A LOCATION TO DETER MOTORISTS FROM SPEEDING. SHOULD IT BE NECESSARY TO LEAVE THE PROJECT SITE, THE LEO SHALL NOTIFY THE ENGINEER. THE CONTRACTOR SHALL PROVIDE THE LEO WITH A TWO-WAY COMMUNICATION DEVICE WHICH SHALL BE RETURNED TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT.

LEOs (WITH PATROL CAR) REQUIRED BY THE TRAFFIC MAINTENANCE TASKS ABOVE SHALL BE PAID FOR ON A UNIT PRICE (HOURLY) BASIS UNDER ITEM 614, LAW ENFORCEMENT OFFICE (WITH PATROL CAR) FOR ASSISTANCE. THE FOLLOWING ESTIMATED QUANTITY HAS BEEN CARRIED TO THE GENERAL SUMMARY.

ITEM 614, LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE 30 HOURS

THE HOURS PAID SHALL INCLUDE ANY MINIMUM SHOW-UP TIME REQUIRED BY THE LAW ENFORCEMENT AGENCY INVOLVED.

ANY ADDITIONAL COSTS (ADMINISTRATIVE OR OTHERWISE) INCURRED BY THE CONTRACTOR TO OBTAIN THE SERVICES OF AN LEO ARE INCLUDED WITH THE BID UNIT PRICE FOR ITEM 614, LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE.

FULLY-ACTUATED OPERATION OF WORK ZONE TRAFFIC SIGNAL
 THE WORK ZONE SIGNAL CONTROL REQUIRED FOR THIS PROJECT AND SHOWN ON SHEETS 7-8 AND STANDARD CONSTRUCTION DRAWINGS MT-96.11, MT-96.20, AND MT-96.26 SHALL BE FULLY TRAFFIC-ACTUATED AND OPERATE IN A MANNER SIMILAR TO THAT DESCRIBED IN SECTION 733.02 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS.

THE INITIAL CONTROLLER TIMING SHALL BE AS FOLLOWS:

	PHASE *			
	1 (ALL RED) DUMMY PHASE	2 MAINLINE (DIRECTION) †	3 (ALL RED) DUMMY PHASE	4 MAINLINE (DIRECTION) †
MIN. GREEN	Tc1 - 6±	10	Tc3 - 6±	10
EXTENSION	N/A	4	N/A	4
MAX. GREEN	Tc1 - 5±	30	Tc3 - 5±	30
YELLOW	3	3.5	3	3.5
ALL RED	2	2	2	2
RECALL	MAX.	MIN.	MAX.	MIN.

*PHASES AS SHOWN ON SCD MT-96.20 FOR ACTUATED CONTROL. ADD MORE PHASES AS NEEDED TO ACCOMMODATE SIDE STREETS, DRIVEWAYS, ETC.

†PROVIDE TIMING FOR THE SIGNAL LOCATION UNDER CONSIDERATION.

Tc1 IS THE DESIRED INTERNAL CLEARANCE TIME FOR PHASE 1. Tc3 IS THE DESIRED INTERNAL CLEARANCE TIME FOR PHASE 3. USUALLY, Tc1 = Tc3.

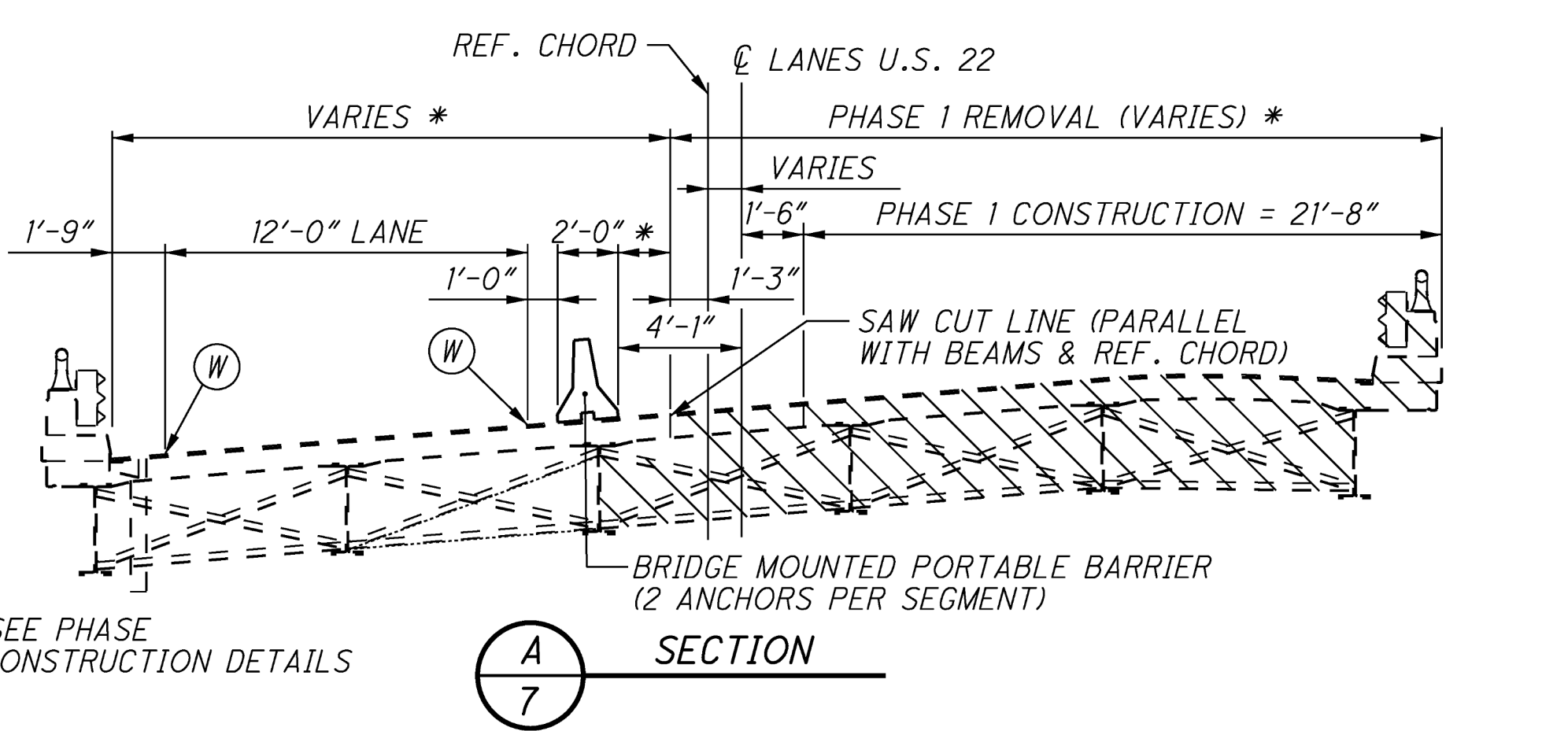
‡INDICATE DIRECTION OF GREEN.

THE CONTRACTOR SHALL ALSO DESIGN, FURNISH, INSTALL AND MAINTAIN A TRAFFIC DETECTOR ON EACH TRAFFIC APPROACH WHICH WILL RELIABLY DETECT ALL LEGAL TRAFFIC APPROACHING (BUT NOT LEAVING) THE SIGNAL AS IT PASSES OR WAITS IN THE DESIGNATED DETECTOR ZONE SHOWN IN THE PLANS. DETECTOR DESIGNS WHICH DO NOT PROVIDE RELIABLE DETECTION, FREE FROM FALSE CALLS, SHALL BE IMMEDIATELY REPLACED BY THE CONTRACTOR.

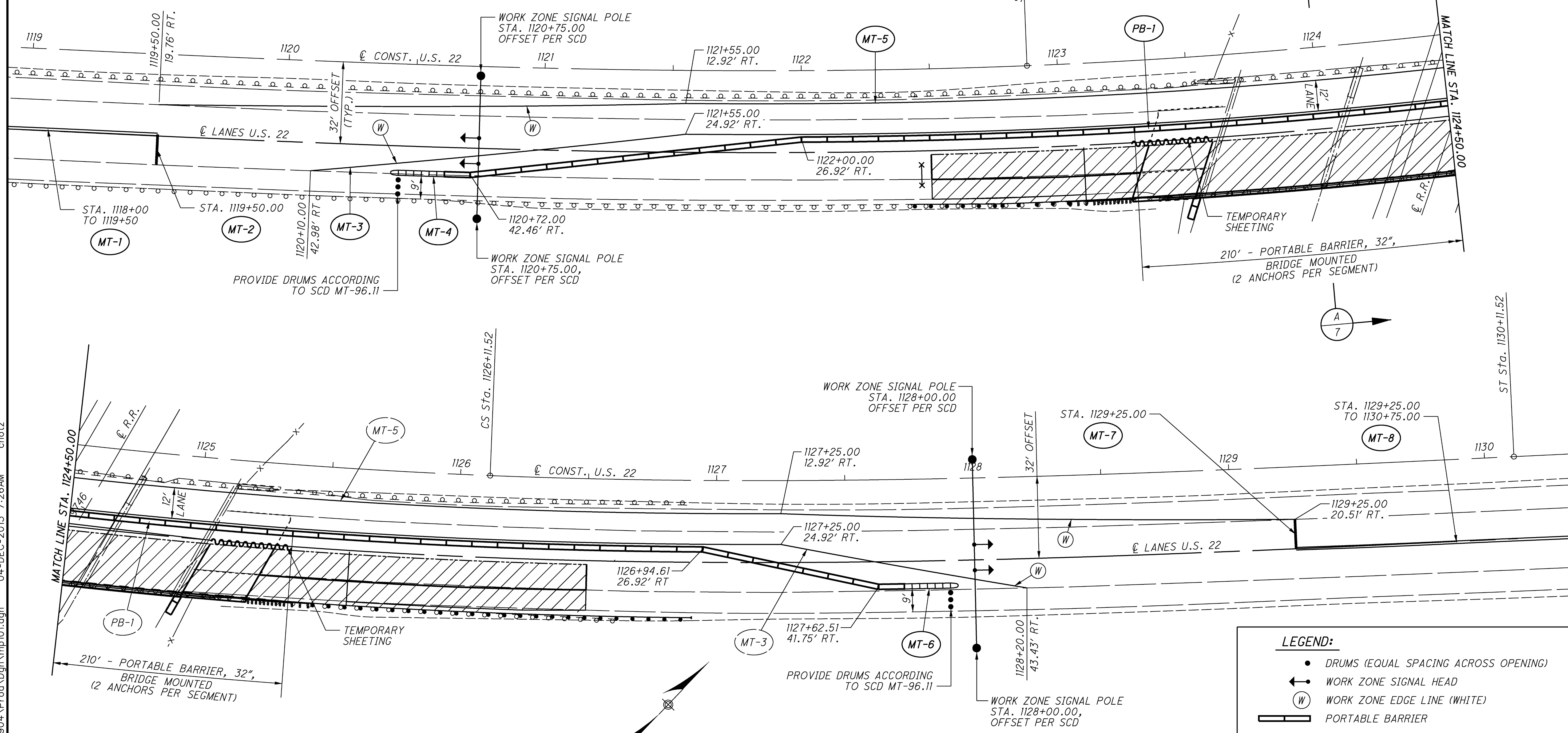
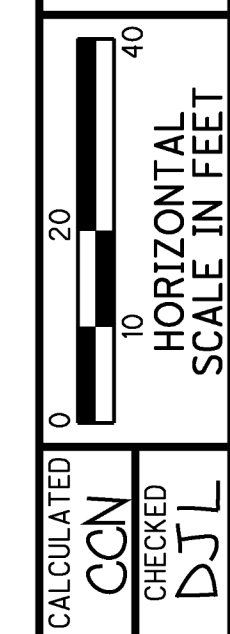
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MAINTENANCE OF TRAFFIC QUANTITIES										
SHEET NO.	REFERENCE	614						622		
		WORK ZONE IMPACT ATTENUATOR (BIDIRECTIONAL) FOR 24" WIDE HAZARDS	BARRIER REFLECTOR, TYPE B2	OBJECT MARKER, TWO WAY	WORK ZONE CENTER LINE, CLASS I, 642 PAINT	WORK ZONE EDGE LINE, CLASS I, 740.06, TYPE I	WORK ZONE STOP LINE, CLASS I, 740.06, TYPE I	PORTABLE BARRIER, 32"	PORTABLE BARRIER, 32", BRIDGE MOUNTED	
		EACH	EACH	EACH	MILE	MILE	FEET	FEET	FEET	
PHASE 1	7	MT-1				0.03				
	7	MT-2						12		
	7	MT-3					0.16			
	7	MT-4	1							
	7	MT-5					0.19			
	7	MT-6	1							
	7	MT-7						12		
	7	MT-8				0.03				
7	PB-1		15	15				510	210	
PHASE 1 SUB-TOTAL			2	15	15	0.06	0.35	24	510	210
PHASE 2	8	MT-9					0.19			
	8	MT-10					0.15			
	8	MT-11	1							
	8	MT-12	1							
	8	PB-2		15	15				480	210
PHASE 2 SUB-TOTAL			2	15	15		0.34		480	210
TOTAL CARRIED TO GENERAL SUMMARY			4	30	30	0.06	0.69	24	990	420

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- NOTES:**
1. FOR DETAILS NOT SHOWN, SEE STANDARD CONSTRUCTION DRAWINGS MT-96.11, MT-96.20, MT-101.70, RM-4.2, AND PCB-91.
 2. ALL OFFSETS ARE MEASURED FROM THE U.S. 22 CENTERLINE OF CONSTRUCTION.
 3. FOR MAINTENANCE OF TRAFFIC QUANTITIES, SEE SHEET NO. 6.
 4. FOR STRUCTURE PHASE CONSTRUCTION DETAILS, SEE SHEET NO. 27.

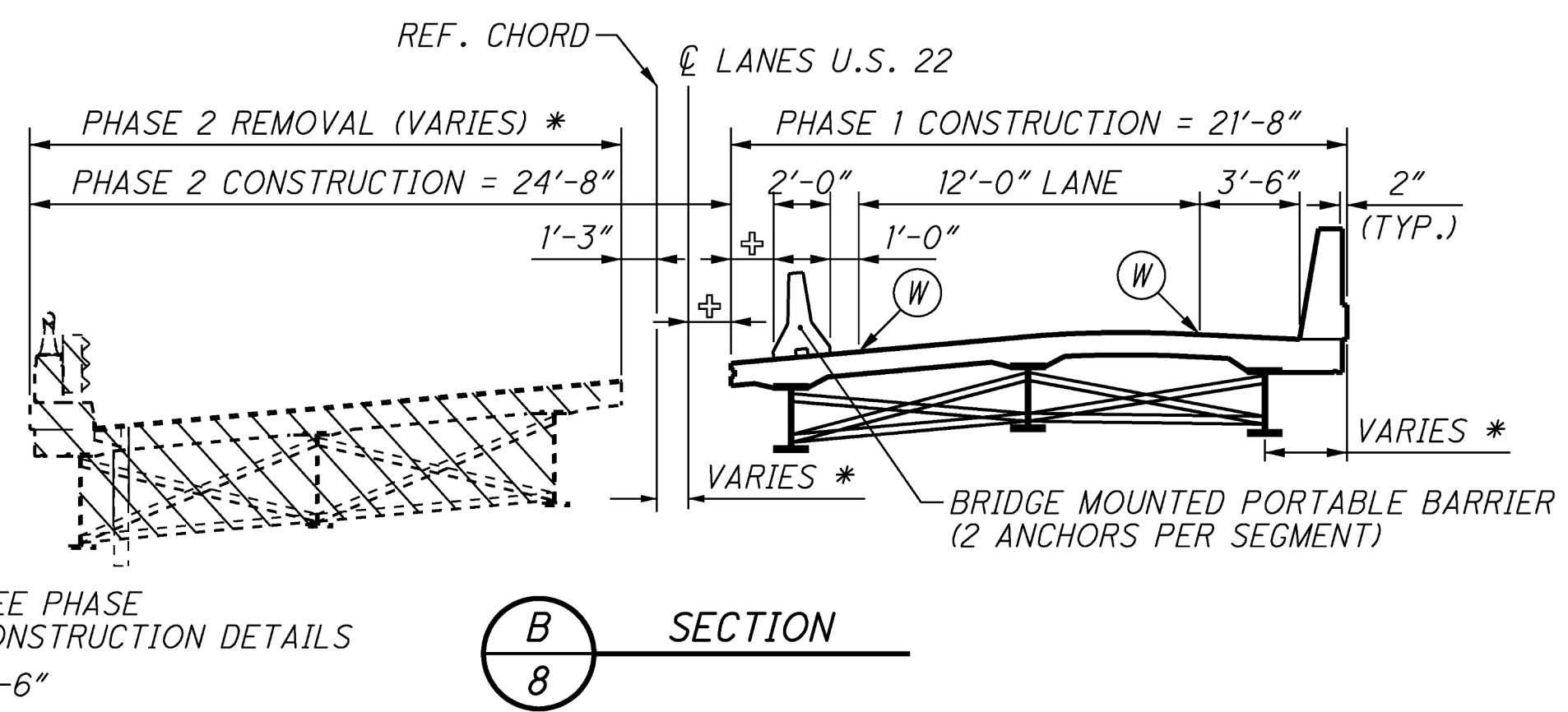


- LEGEND:**
- DRUMS (EQUAL SPACING ACROSS OPENING)
 - ↕ WORK ZONE SIGNAL HEAD
 - (W) WORK ZONE EDGE LINE (WHITE)
 - ▬ PORTABLE BARRIER
 - ▬ WORK ZONE IMPACT ATTENUATOR
 - ××× TYPE 3 BARRICADE
 - ▨ PHASE 1 CONSTRUCTION

MAINTENANCE OF TRAFFIC PLAN PHASE 1

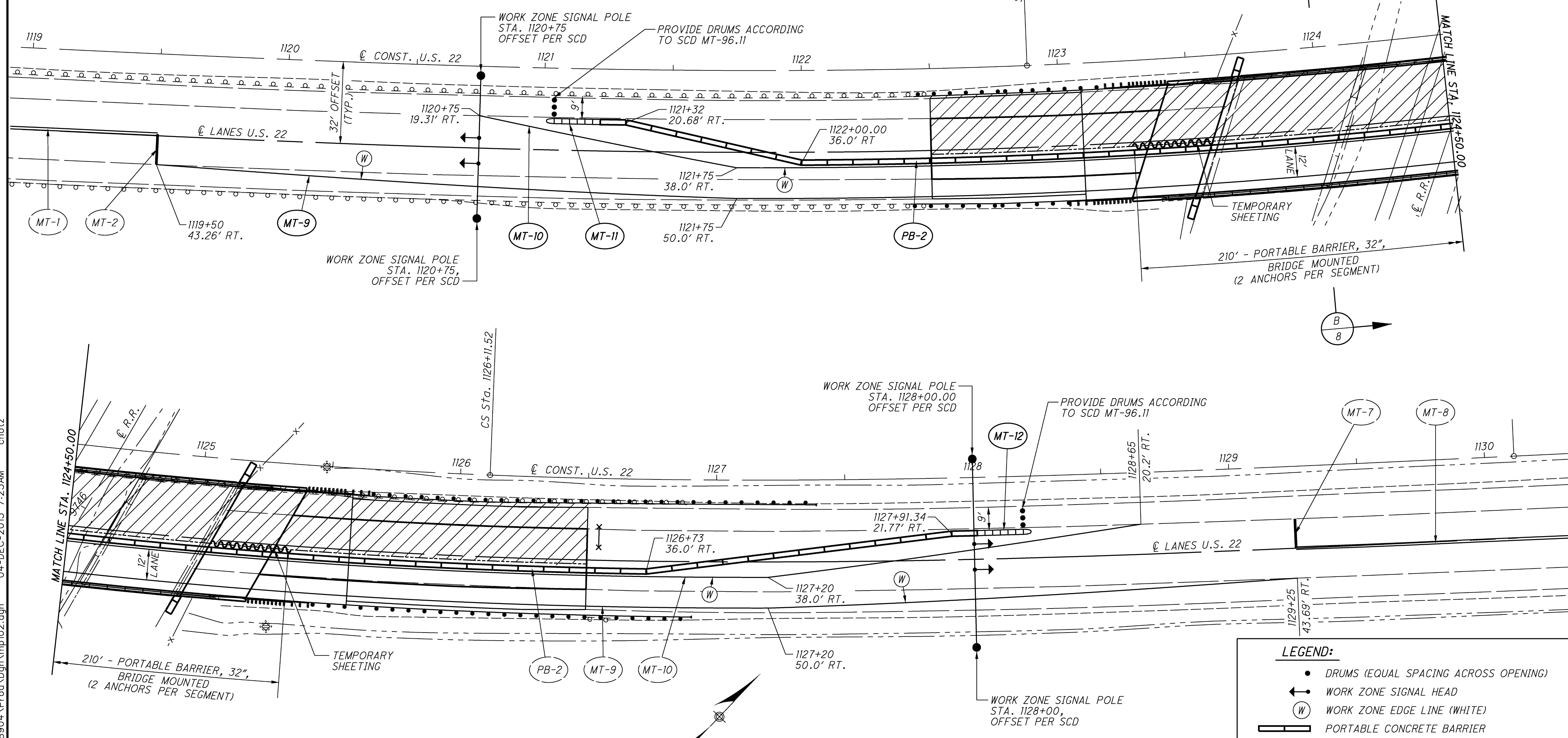
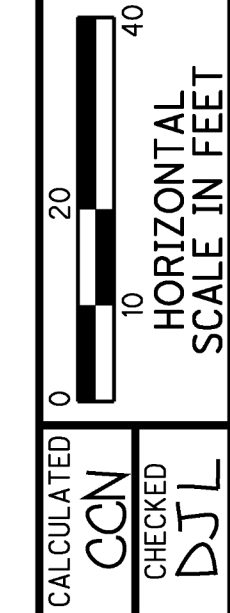
HAS-22-21.26

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NOTES:

1. FOR DETAILS NOT SHOWN, SEE STANDARD CONSTRUCTION DRAWINGS MT-96.11, MT-96.20, MT-101.70, RM-4.2, AND PCB-91.
2. ALL OFFSETS ARE MEASURED FROM THE U.S. 22 CENTERLINE OF CONSTRUCTION.
3. FOR MAINTENANCE OF TRAFFIC QUANTITIES, SEE SHEET NO. 6.
4. FOR STRUCTURE PHASE CONSTRUCTION DETAILS, SEE SHEET NO. 27.



LEGEND:

- DRUMS (EQUAL SPACING ACROSS OPENING)
- ← WORK ZONE SIGNAL HEAD
- (W) WORK ZONE EDGE LINE (WHITE)
- ▬ PORTABLE CONCRETE BARRIER
- ▬ WORK ZONE IMPACT ATTENUATOR
- x-x TYPE 3 BARRICADE
- ▨ PHASE 2 CONSTRUCTION

MAINTENANCE OF TRAFFIC PLAN PHASE 2

HAS-22-21.26

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SHEET NUMBER											01/ NHS/ BR	ITEM	ITEM EXT.	GRAND TOTAL	UNIT	DESCRIPTION	SEE SHEET NO.		
5	6				49	50	51	52	OFFICE CALCS										
										LUMP	LUMP	202	11203	LUMP		STRUCTURES (20' AND OVER)			
										133	133	202	22900	133	SQ YD	PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN APPROACH SLAB REMOVED	25		
										LUMP	LUMP	503	11101	LUMP		COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN	25		
										LUMP	LUMP	503	21300	LUMP		UNCLASSIFIED EXCAVATION			
										41	41	503	31120	41	CU YD	SHALE EXCAVATION			
										LUMP	LUMP	505	11100	LUMP		PILE DRIVING EQUIPMENT MOBILIZATION			
										210	210	507	00200	210	FT	STEEL PILES HP12X53, FURNISHED			
										180	180	507	00250	180	FT	STEEL PILES HP12X53, DRIVEN			
					9230	1603	8024	52454				71311	509	10000	71311	POUND	EPOXY COATED REINFORCING STEEL		
												203	203	510	10000	203	EACH	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT	
												240	240	511	34447	240	CU YD	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK, AS PER PLAN	25
												47	47	511	34450	47	CU YD	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET)	
												55	55	511	45712	55	CU YD	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT	
												21	21	511	46512	21	CU YD	CLASS QC1 CONCRETE WITH QC/QA, FOOTING	
												904	904	512	10100	904	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	
												24	24	512	33000	24	SQ YD	TYPE 2 WATERPROOFING	
												137244	137244	513	10240	137244	POUND	STRUCTURAL STEEL MEMBERS, LEVEL 2	
												3276	3276	513	20000	3276	EACH	WELDED STUD SHEAR CONNECTORS	
												16	16	516	13600	16	SQ FT	1" PREFORMED EXPANSION JOINT FILLER	
												38	38	516	13900	38	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	
												129	129	516	14020	129	FT	SEMI-INTEGRAL ABUTMENT EXPANSION JOINT SEAL	
												12	12	516	44201	12	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (10" x 14" x 3.3" PAD WITH 11" X 15" X 1 1/2" LOAD PLATE AND HP PEDESTALS)	36
												12	12	516	44201	12	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (10" x 18" x 3.3" PAD WITH 11" X 19" X 2" LOAD PLATE, 11" X 14" X 2" CAP PLATE, AND HP PEDESTALS)	38
												LUMP	LUMP	518	21230	LUMP		POROUS BACKFILL WITH FILTER FABRIC	
												125	125	518	40000	125	FT	6" PERFORATED CORRUGATED PLASTIC PIPE	
												34	34	518	40010	34	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	
												LUMP	LUMP	518	63300	LUMP		STRUCTURE DRAINAGE, MISC.: PLUG EXISTING 4"+/- WEEP HOLES	25
												256	256	526	25001	256	SQ YD	REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN	48
												4	4	601	20000	4	SQ YD	CRUSHED AGGREGATE SLOPE PROTECTION	
												274	274	601	28000	274	CU YD	DUMPED ROCK FILL, TYPE D	
																		MAINTENANCE OF TRAFFIC	
	30											30	614	11110	30	hour	LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE		
		4										4	614	12348	4	EACH	WORK ZONE IMPACT ATTENUATOR (BIDIRECTIONAL), FOR 24" WIDE HAZARDS		
	21											21	614	13202	21	EACH	BARRIER REFLECTOR, TYPE A2		
		30										30	614	13302	30	EACH	BARRIER REFLECTOR, TYPE B2		
		30										30	614	13360	30	EACH	OBJECT MARKER, TWO WAY		
		0.06										0.06	614	21100	0.06	MILE	WORK ZONE CENTER LINE, CLASS I, 642 PAINT		
		0.69										0.69	614	22200	0.69	MILE	WORK ZONE EDGE LINE, CLASS I, 740.06, TYPE 1		
		24										24	614	26400	24	FT	WORK ZONE STOP LINE, CLASS I, 740.06, TYPE 1		
	5											5	616	10000	5	M GAL	WATER		
		990										990	622	41000	990	FT	PORTABLE BARRIER, 32"		
		420										420	622	41020	420	FT	PORTABLE BARRIER, 32", BRIDGE MOUNTED		
												LUMP	LUMP	614	11000	LUMP		MAINTAINING TRAFFIC	
												4	619	16010	4	MONTH	FIELD OFFICE, TYPE B		
												LUMP	LUMP	623	10000	LUMP		CONSTRUCTION LAYOUT STAKES AND SURVEYING	
												LUMP	LUMP	624	10000	LUMP		MOBILIZATION	

GENERAL SUMMARY

HAS - 22 - 21.26

CALCULATED
CCN
CHECKED
DJL

ESTIMATED QUANTITIES

HAS - 22 - 21.26

REFERENCE	PLAN SHEET	STATION		ITEM			202		601	606			607		609	626		630		
				SIDE	LENGTH FT.	WIDTH FT.	GUARDRAIL REMOVED FEET	FENCE REMOVED FEET	TIED CONCRETE BLOCK MAT, TYPE 2 SQ. YD.	GUARDRAIL, TYPE MGS WITH LONG POSTS FEET	ANCHOR ASSEMBLY, MGS TYPE E EACH	MGS BRIDGE TERMINAL ASSEMBLY, TYPE 1 EACH	FENCE, TYPE 47 FEET	FENCELINE SEEDING AND MULCHING FEET	CURB, TYPE 4-C FEET	BARRIER REFLECTOR		GROUND MOUNTED SUPPORT, NO. 3 POST FEET	REMOVAL OF GROUND MOUNTED SIGN AND REERECTION EACH	REMOVAL OF GROUND MOUNTED POST SUPPORT AND DISPOSAL EACH
																A2	B2			
R-1	13	1122+43.6 +/-	1123+50.8 +/-	LT.			107.9 +/-													
R-2	13	1122+41.5 +/-	1123+36.0 +/-	RT.			96.5 +/-													
R-3	13	1125+31.3 +/-	1126+88.5 +/-	LT.			157.9 +/-													
R-4	13	1125+12.7 +/-	1126+60.7 +/-	RT.			151.4 +/-													
F-1	13	1123+60.76 +/-	1123+74.94 +/-	LT.				32.4 +/-				22.16	22.16							
F-2	13	1123+39.96 +/-	1123+46.94 +/-	RT.				25.3 +/-				19.17	19.17							
F-3	13	1125+17.05 +/-	1125+32.67 +/-	LT.				24.3 +/-				17.12	17.12							
F-4	13	1124+92.34 +/-	1124+99.79 +/-	RT.				15.0 +/-				7.14	7.14							
GR-1	13	1122+43.55	1123+45.00	LT.						75.00		1								
GR-2	13	1122+41.52	1123+28.94	RT.						62.50		1								
GR-3	13	1125+38.00	1127+39.09	LT.						125.00	1	1								
GR-4	13	1125+17.74	1126+90.78	RT.						100.00	1	1								
C-1	13	1123+24.44	1123+42.51	LT.	18.10										18.10					
C-2	13	1123+08.77	1123+26.49	RT.	18.10										18.10					
C-3	13	1125+40.49	1125+58.56	LT.	18.10										18.10					
C-4	13	1125+20.19	1125+37.92	RT.	18.10										18.10					
D-1	13	1123+24.44		LT.	51.00	9.00			51											
S-1	13	1125+63.53		LT.														13.9	1	1
S-2	13	1125+59.16		RT.														13.5	1	1
		1122+43.55	1127+39.09	LT.												5	2			
		1122+41.52	1126+90.78	RT.												5	2			
TOTALS		CARRIED TO GENERAL SUMMARY						514	97	51	362.5	2	4	66	66	72	14	27.4	2	2

ALL QUANTITIES CARRIED TO GENERAL SUMMARY

PAVEMENT PLANING AND WEDGE TREATMENT TABLE

254	PAVEMENT PLANING END WIDTH	LEFT SHOULDER OFFSET	ELEVATION AT LEFT EDGE OF LEFT SHOULDER		STATION	OFFSET & SIDE FROM CL WHERE PLANING AND PAVEMENT WEDGE = 0			ELEVATION AT RIGHT EDGE OF RIGHT SHOULDER		RIGHT SHOULDER OFFSET	ASPHALT WEDGE END AREA	448
			EXISTING	PROPOSED		OFFSET (FT.)	SIDE	EXISTING ELEVATION	EXISTING	PROPOSED			FT.
SQ. YD.	FT.	FT.	EXISTING	PROPOSED		OFFSET (FT.)	SIDE	EXISTING ELEVATION	EXISTING	PROPOSED	FT.	SQ. FT.	CU. YD.
--	39.21	20.30 +/-	1150.43 +/-	1150.43	1122+50.00	--	--	--	1152.33 +/-	1152.33	18.91 +/-	0	--
102.10	34.30	20.67	1150.58 +/-	1150.54	1122+75.00	13.63	RT.	1152.78 +/-	1152.61 +/-	1153.00	19.25	0.47	0.22
93.19	32.80	20.67	1150.72 +/-	1150.78	1123+00.00	12.13	RT.	1153.05 +/-	1152.89 +/-	1153.42	19.25	1.39	0.86
32.03	32.95	20.67	1150.74 +/-	1150.87	1123+08.77	12.28	RT.	1153.19 +/-	1152.99 +/-	1153.55	19.25	1.58	0.48
--	29.96	20.00	1153.38 +/-	1153.32	1125+58.56	9.96	RT.	1155.50 +/-	1155.54 +/-	1156.06	20.00	2.12	--
56.18	31.55	20.00	1153.61 +/-	1153.47	1125+75.00	11.55	RT.	1155.78 +/-	1155.7 +/-	1156.21	20.00	1.88	1.22
88.75	32.35	20.00	1153.84 +/-	1153.71	1126+00.00	12.35	RT.	1155.99 +/-	1155.9 +/-	1156.35	20.00	1.04	1.35
91.22	33.33	20.00	1154.05 +/-	1153.95	1126+25.00	13.33	RT.	1156.20 +/-	1156.09 +/-	1156.45	20.00	0.31	0.63
101.72	39.91 +/-	20.58 +/-	1154.18 +/-	1154.18	1126+50.00	--	--	--	1156.38 +/-	1156.38	19.33 +/-	0	0.14
565	TOTALS (CARRIED TO GENERAL SUMMARY)												5

**ALL OFFSETS ARE MEASURED WITH RESPECT TO C LINES

FROM SHEET NO.	203		659
	EXCAVATION	EMBANKMENT	SEEDING & MULCHING
	CU. YD.	CU. YD.	SQ. YD.
14	0	1	126
15	0	4	130
16	88	65	240
17	53	16	116
18	60	15	177
19	0	8	209
20	0	9	297
21	0	3	81
23	0	0	20
TOTAL	201	121	1396

ITEM 621 - RPM
 STA. 1122+50.00 TO STA. 1123+34.42
 85.58 FT. ÷ 40 FT. SPACING = 2.14 + 1 = 3.14 (USE 4 EACH)
 STA. 1125+30.23 TO STA. 1126+50.00
 121.41 FT. ÷ 40 FT. SPACING = 3.04 + 1 = 4.04 (USE 5 EACH)
TOTAL = 9 EACH

ITEM 621 - RAISED PAVEMENT MARKER REMOVED
 STA. 1122+50.00 TO STA. 1126+50.00
 405.5 FT. ÷ 40 FT. SPACING = 10.14 + 1 = 11.14 EACH (USE 12 EACH)

ITEM 642 - EDGE LINE, 4", TYPE 1
 STA. 1122+50.00 TO STA. 1126+50.00 (RT. & LT.)
 405.5 FT. ÷ 5280 = 0.08 * 2 SIDES = 0.16 MILES (USE 0.16 MILES)

ITEM 642 - CENTER LINE, TYPE 1
 STA. 1122+50.00 TO STA. 1126+50.00
 405.5 FT. ÷ 5280 = 0.08 MILES (USE 0.08 MILES)

ITEM 659 - SOIL ANALYSIS TEST
 155 C.Y. x 1 TEST/10000 C.Y. = 0.016 EACH (MINIMUM OF 2 TESTS) (USE 2 EACH)

ITEM 659 - TOPSOIL
 1396 S.Y. x 111 C.Y./1000 S.Y. = 154.96 C.Y. (USE 155 C.Y.)

ITEM 659 - REPAIR SEEDING AND MULCHING
 1396 S.Y. x 0.05 = 69.8 S.Y. (USE 70 S.Y.)

ITEM 659 - INTER-SEEDING
 1396 S.Y. x 0.05 = 69.8 S.Y. (USE 70 S.Y.)

ITEM 659 - LIME
 1396 S.Y. x (9 S.F./S.Y.)/(43,560 S.F./ACRE) = 0.29 ACRES (USE 0.29 ACRES)

ITEM 659 - WATER
 1396 S.Y. x 9 x 300 GAL/1000 S.F. x 2 APP./1000 S.F. = 7.54 M. GAL.
 (FOR INTER-SEEDING)
 70 S.Y. x 9 x 300 GAL/1000 S.F. x 2 APP./1000 S.F. = 0.19 M. GAL.
TOTAL = 7.73 M. GAL.

ITEM 659 - COMMERCIAL FERTILIZER
 1396 S.Y. x 9 x 30 LBS/1000 S.F. / 2000 LB./TON = 0.19 TON
 (FOR INTER-SEEDING)
 70 S.Y. x 9 x 30 LBS/1000 S.F. / 2000 LB./TON = 0.006 TON
TOTAL = 0.20 TON

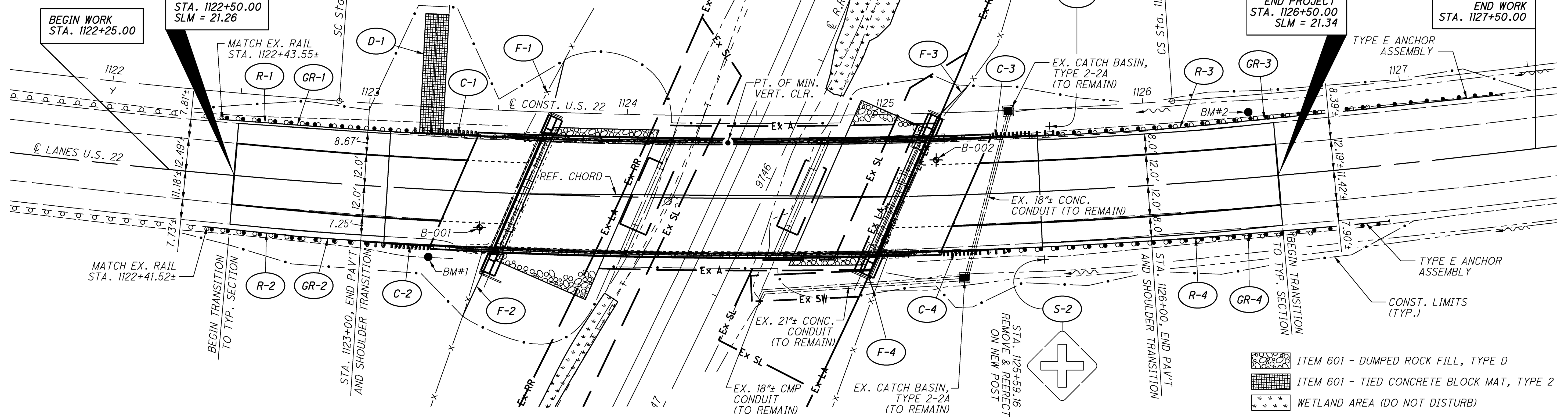
ALL QUANTITIES CARRIED TO GENERAL SUMMARY

P.I. Sta. 1124+53.30 $y = 11.63'$ $C = 323.06'$
 $\Delta = 18^\circ 05' 00''$ (LT) $k = 199.95'$ $e_{max} = 0.081$
 $Dc = 2^\circ 30' 00''$ $p = 2.91'$ $T.S. = 1118+88.19$
 $R = 2,291.83'$ $\Delta c = 8^\circ 05' 00''$ (LT) $S.T. = 1130+11.52$
 $Ls = 400.00'$ $Lc = 323.33'$ $E = 31.78'$
 $\theta s = 5^\circ 00' 00''$
 $LT = 266.77'$
 $ST = 133.43'$
 $x = 399.70'$

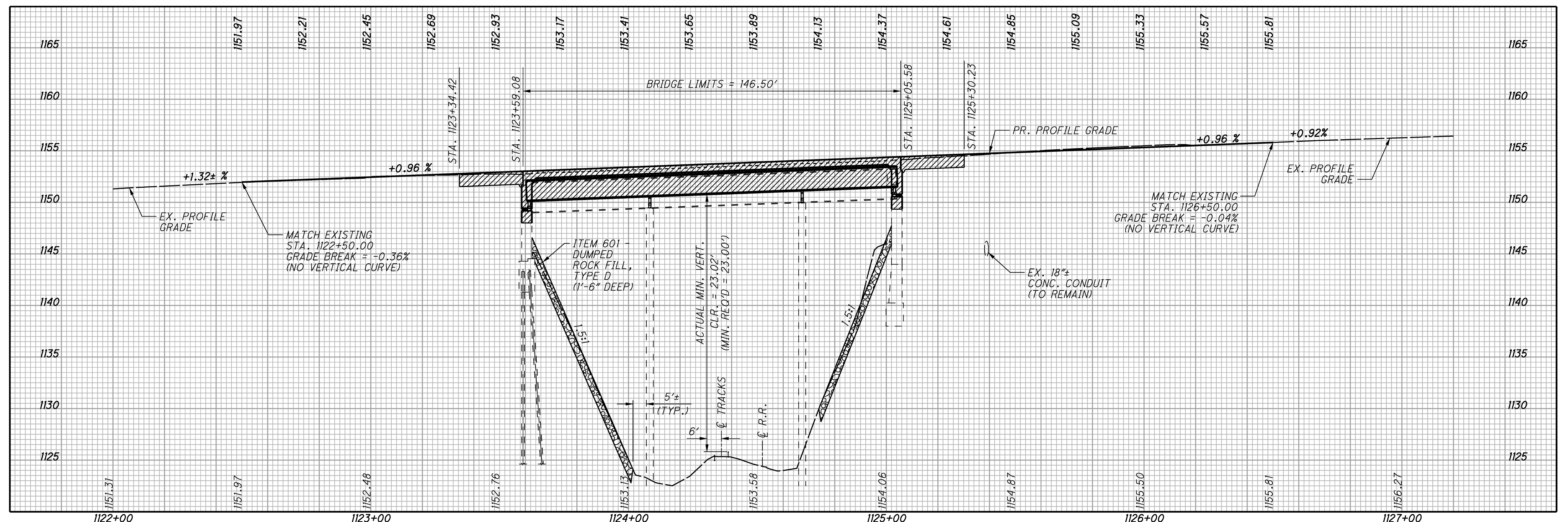
CENTERLINE REFERENCES U.S. 22

<p>CONST. U.S. 22</p> <p>I-PIN SET</p> <p>17.87'</p> <p>12.97'</p> <p>14.76'</p> <p>SURVEY NAIL</p> <p>SURVEY NAIL</p> <p>SURVEY NAIL</p> <p>S.C. STA. 1122+88.19</p>	<p>CONST. U.S. 22</p> <p>I-PIN SET</p> <p>19.52'</p> <p>14.03'</p> <p>29.81'</p> <p>SURVEY NAIL</p> <p>SURVEY NAIL</p> <p>SURVEY NAIL</p> <p>C.MON</p> <p>C.S. STA. 1126+11.52</p>
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<p>BENCH MARK #1</p> <p>STA. 1123+25.88 @ U.S. 22</p> <p>58.17' RT.</p> <p>ELEV. 1152.76</p> <p>CONCRETE MONUMENT WITH ODOT CAP</p>	<p>BENCH MARK #2</p> <p>STA. 1126+40.57 @ U.S. 22</p> <p>6.53' RT.</p> <p>ELEV. 1153.26</p> <p>CONCRETE MONUMENT WITH ODOT CAP</p>
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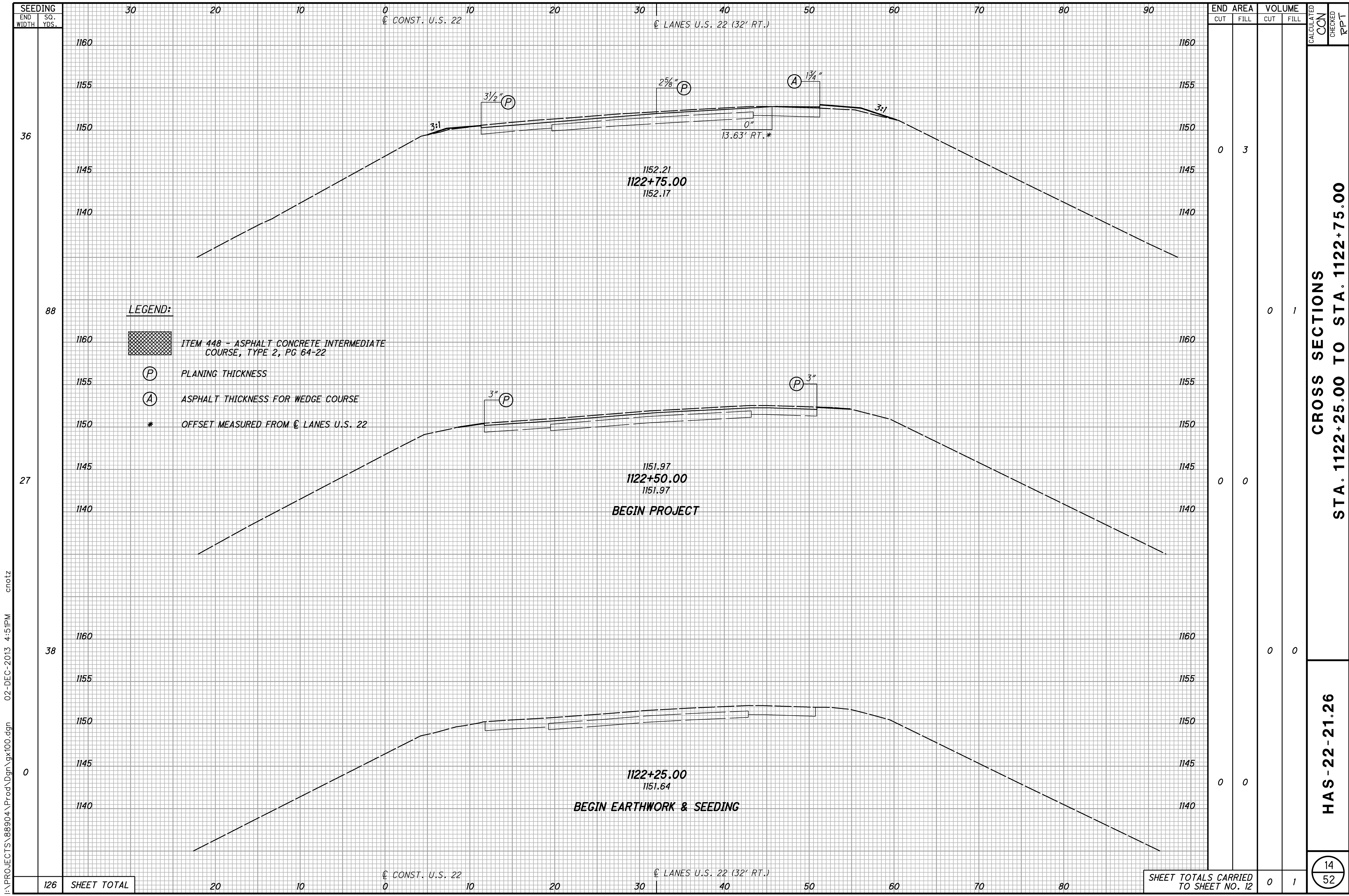


- ITEM 601 - DUMPED ROCK FILL, TYPE D
- ITEM 601 - TIED CONCRETE BLOCK MAT, TYPE 2
- WETLAND AREA (DO NOT DISTURB)



PLAN AND PROFILE
STA. 1121+50.00 TO STA. 1127+50.00

HAS-22-21.26



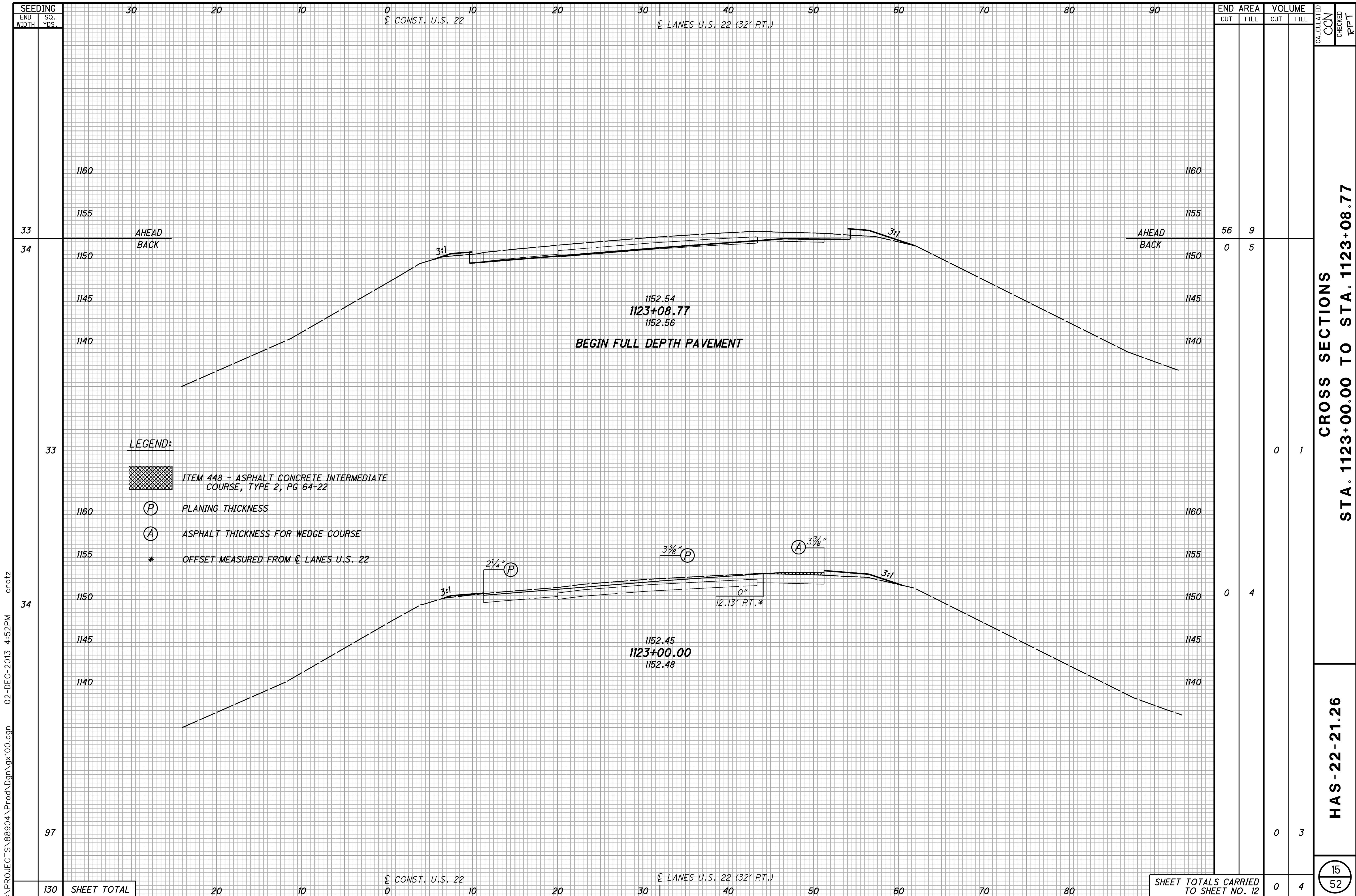
END AREA	VOLUME	CALCULATED		CHECKED	RPT
		CUT	FILL		
0	3	0	1		
0	0	0	0		
0	0	0	0		
SHEET TOTALS CARRIED TO SHEET NO. 12		0	1		

**CROSS SECTIONS
STA. 1122+25.00 TO STA. 1122+75.00**

HAS - 22 - 21.26

14 / 52

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SEEDING	
END WIDTH	SO. YDS.
33	
34	
33	
34	
97	
130	SHEET TOTAL

END AREA		VOLUME		CALCULATED CCN	CHECKED RPT
CUT	FILL	CUT	FILL		
56	9				
0	5				
		0	1		
0	4				
		0	3		
		0	4		

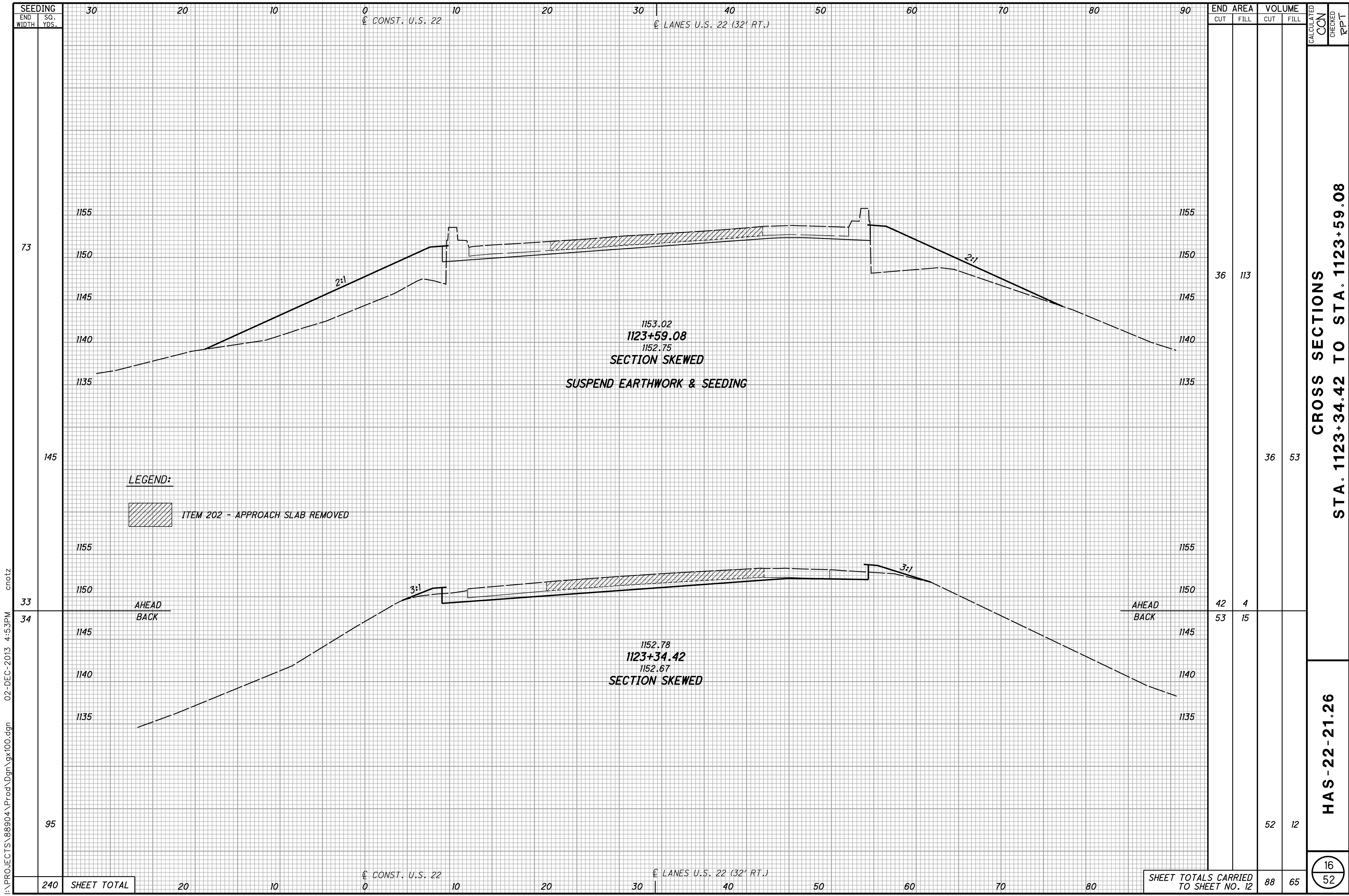
CROSS SECTIONS
 STA. 1123+00.00 TO STA. 1123+08.77

HAS - 22 - 21.26

15
 52

SHEET TOTALS CARRIED TO SHEET NO. 12

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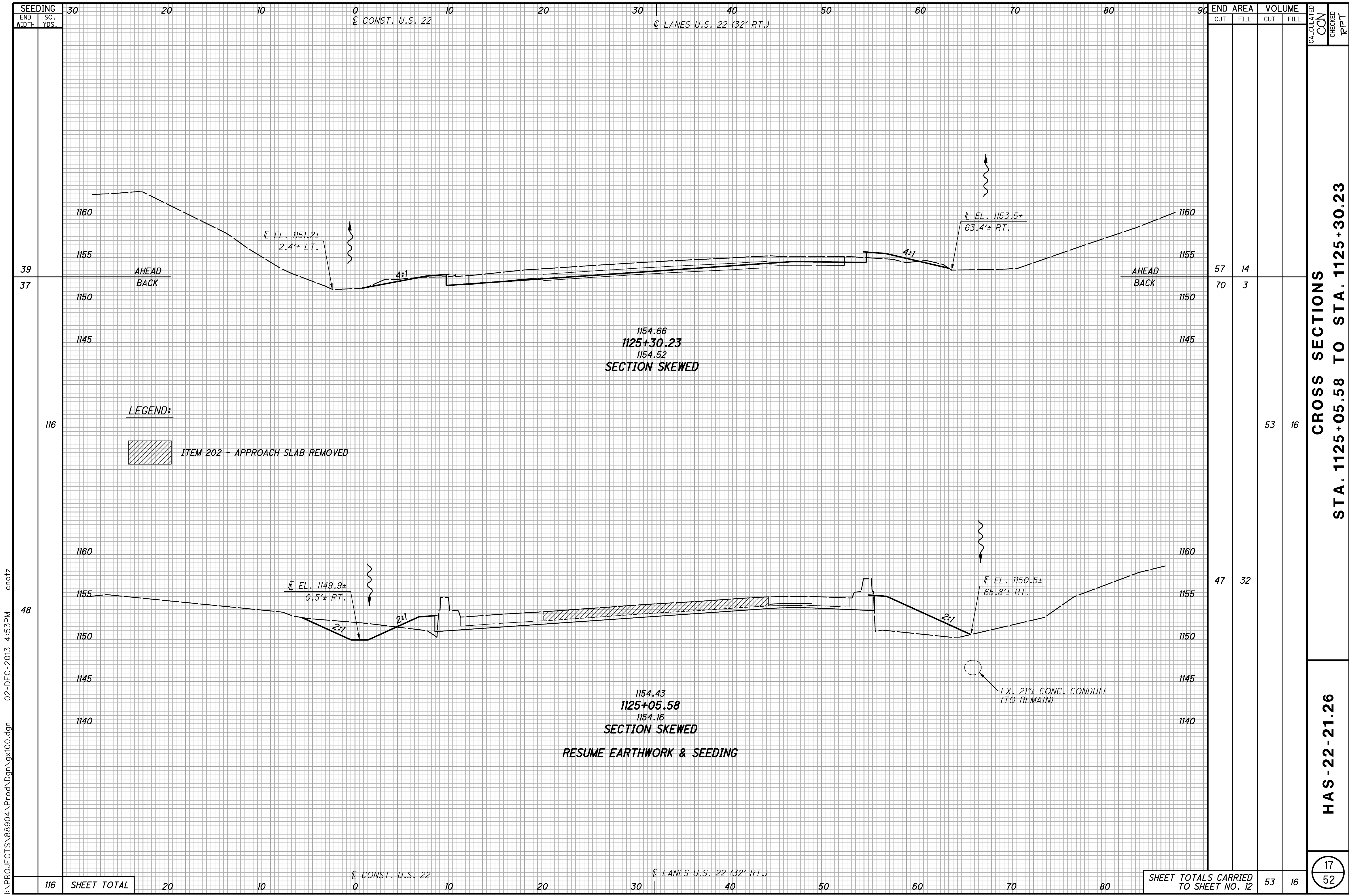
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SEEDING	30		20		10		0		10		20		30		40		50		60		70		80		90	
	END WIDTH	SO. YDS.																								
73																										
145																										
33																										
34																										
95																										
240	SHEET TOTAL		20	10	0	10	20	30	40	50	60	70	80													

END AREA	VOLUME		CALCULATED CCN	CHECKED RPT
	CUT	FILL		
36	113			
36	53			
42	4			
53	15			
	52	12		
SHEET TOTALS CARRIED TO SHEET NO. 12		88	65	

CROSS SECTIONS
STA. 1123+34.42 TO STA. 1123+59.08

HAS-22-21.26



SEEDING
END SO.
WIDTH YDS.

30 20 10 0 10 20 30 40 50 60 70 80 90

39
37
116
48
116

1160
1155
1150
1145
1140

1160
1155
1150
1145
1140

1160
1155
1150
1145
1140

1160
1155
1150
1145
1140

END AREA	VOLUME		CALCULATED CCN	CHECKED RPT
	CUT	FILL		
57	14			
70	3			
		53	16	
47	32			
SHEET TOTALS CARRIED TO SHEET NO. 12		53	16	

CROSS SECTIONS
STA. 1125+05.58 TO STA. 1125+30.23

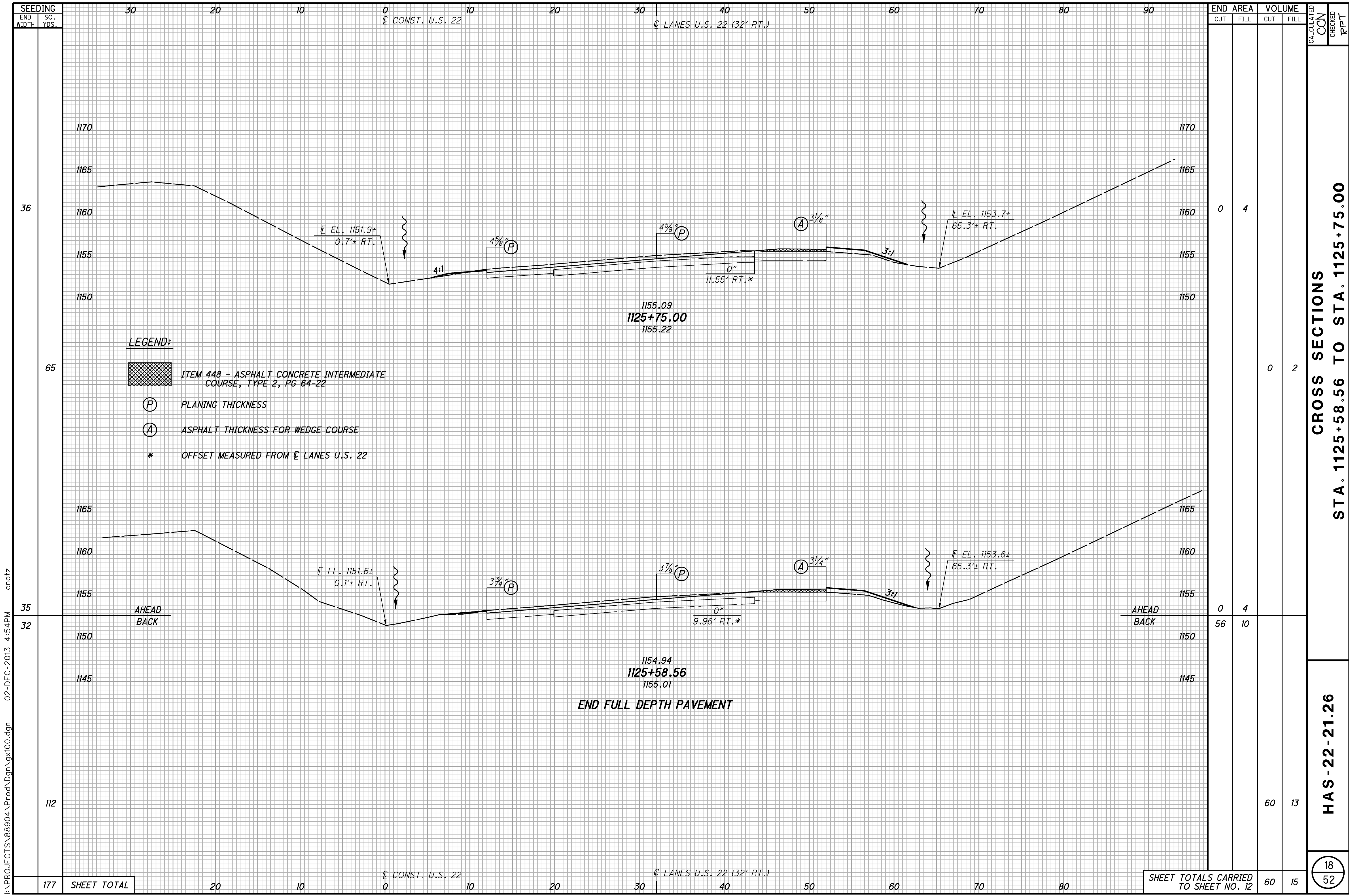
HAS-22-21.26

17
52

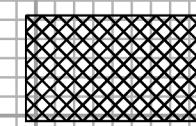


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CONST. U.S. 22 LANES U.S. 22 (32' RT.)

SHEET TOTALS CARRIED TO SHEET NO. 12



LEGEND:

-  ITEM 448 - ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG 64-22
-  PLANING THICKNESS
-  ASPHALT THICKNESS FOR WEDGE COURSE
- * OFFSET MEASURED FROM ϕ LANES U.S. 22

SEEDING	END	
	WIDTH	SO. YDS.
36		
65		
35		
32		
112		
177		

END AREA	VOLUME		CALCULATED	CHECKED	RPT
	CUT	FILL			
0	4				
0	2				
0	4				
56	10				
60	13				
60	15				

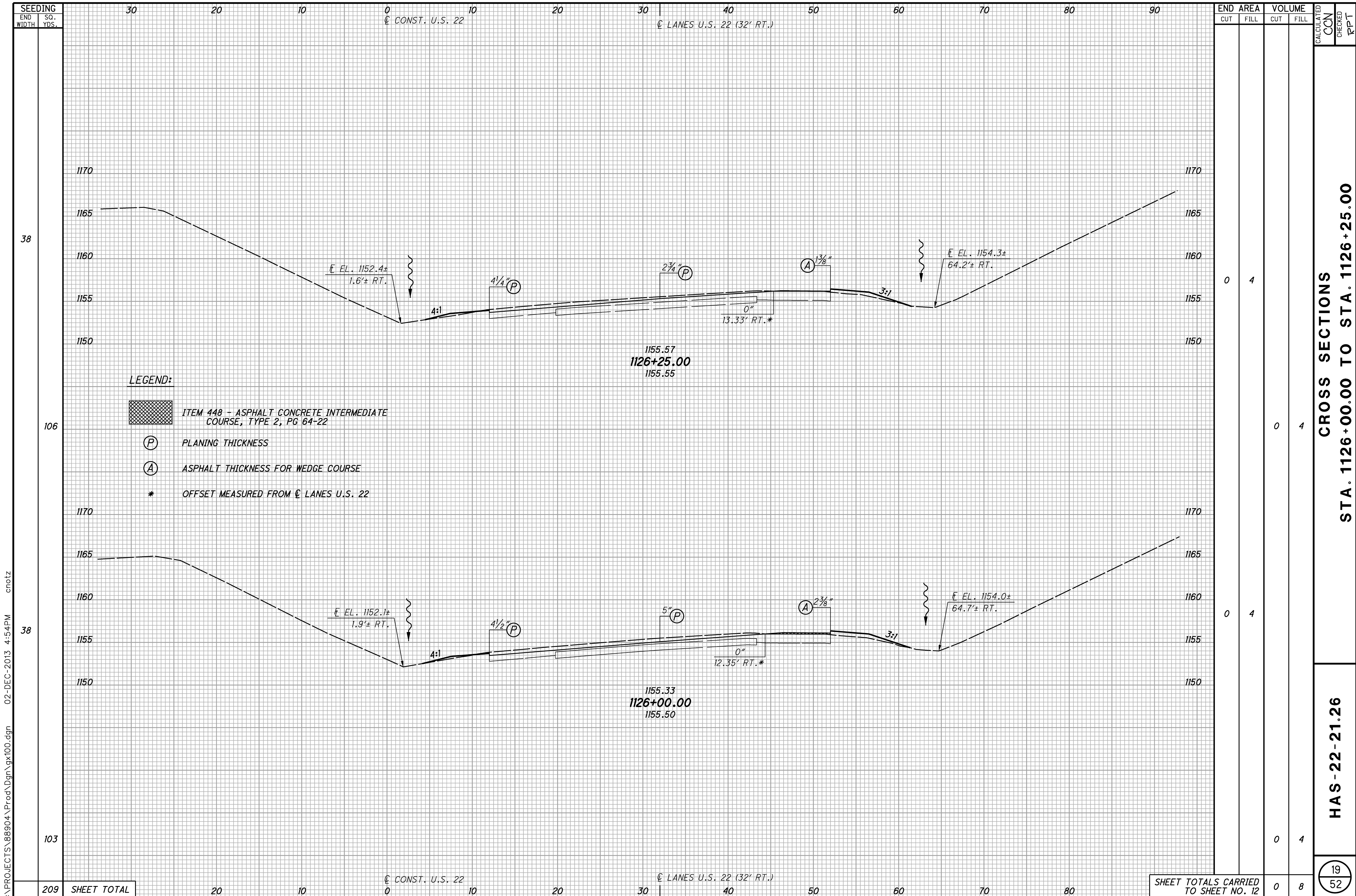
**CROSS SECTIONS
STA. 1125+58.56 TO STA. 1125+75.00**

HAS-22-21.26

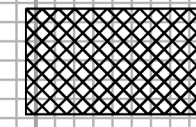


18
52

SHEET TOTALS CARRIED TO SHEET NO. 12

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LEGEND:

-  ITEM 448 - ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, PG 64-22
-  PLANING THICKNESS
-  ASPHALT THICKNESS FOR WEDGE COURSE
- * OFFSET MEASURED FROM $\text{\textcircled{C}}$ LANES U.S. 22

SEEDING	END	
	WIDTH	SO. YDS.
38		
106		
38		
103		
209	SHEET TOTAL	

END AREA	VOLUME		CALCULATED CCN	CHECKED RPT
	CUT	FILL		
0	4	0	4	
0	4	0	4	
0	8	0	8	

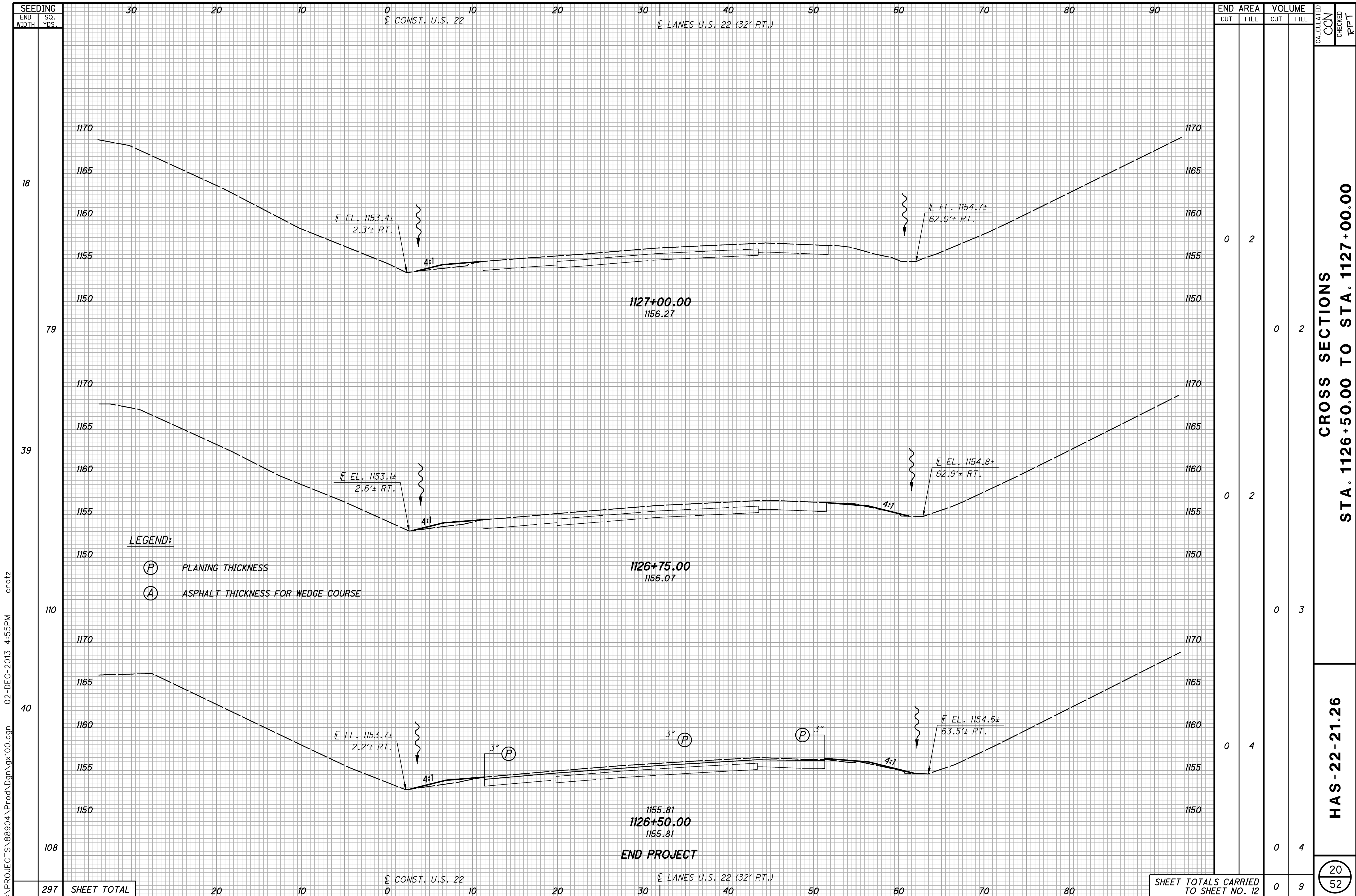
**CROSS SECTIONS
STA. 1126+00.00 TO STA. 1126+25.00**

HAS-22-21.26

19
52

SHEET TOTALS CARRIED TO SHEET NO. 12

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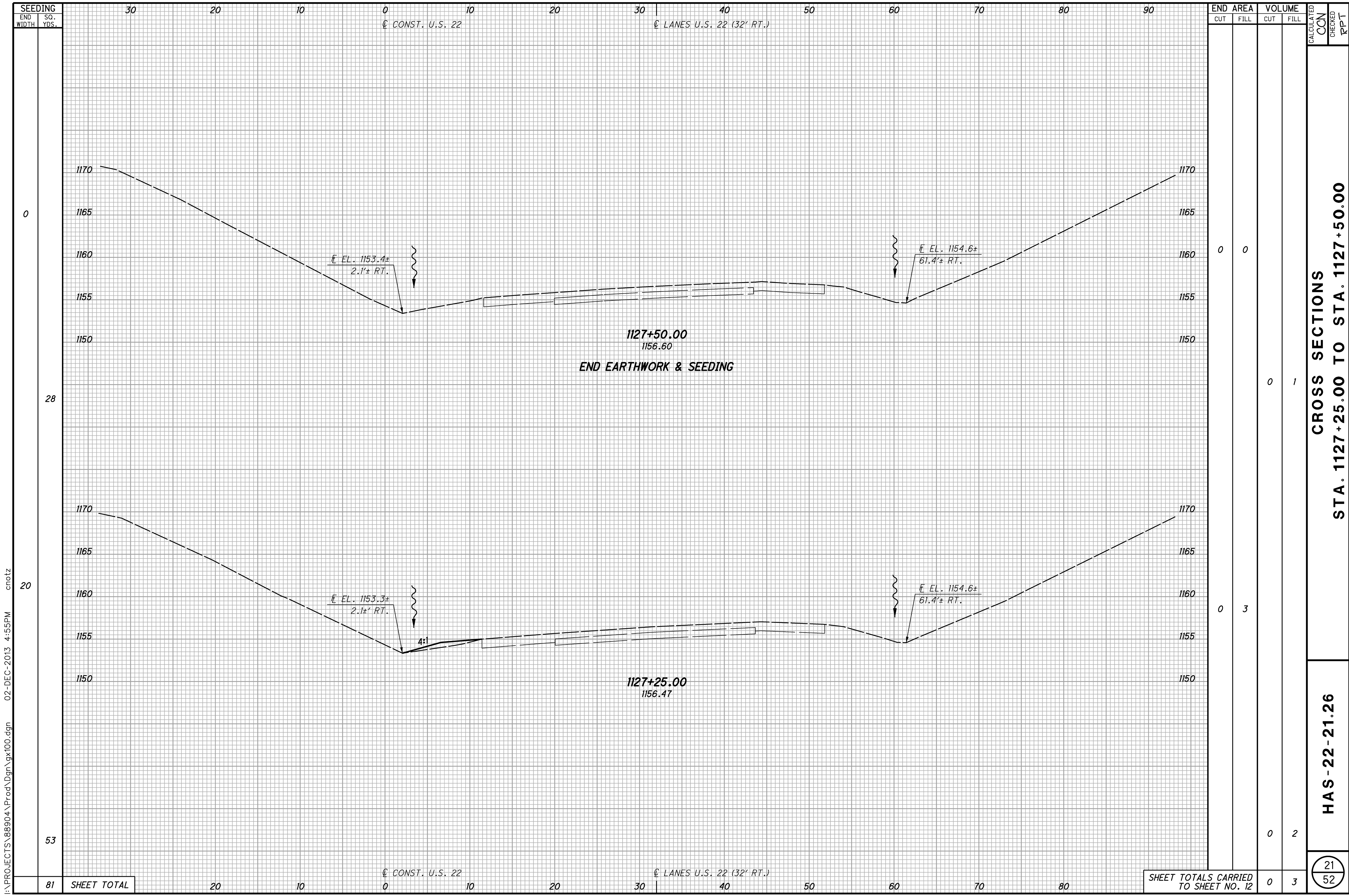
END AREA		VOLUME		CALCULATED CCN	CHECKED RPT
CUT	FILL	CUT	FILL		
0	2	0	2		
0	2	0	2		
0	2	0	3		
0	4	0	4		
0	9	0	9		

**CROSS SECTIONS
STA. 1126+50.00 TO STA. 1127+00.00**

HAS-22-21.26

20
52

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END AREA		VOLUME		CALCULATED CCN	CHECKED RPT
CUT	FILL	CUT	FILL		
0	0	0	1		
0	3	0	2		
0	0	0	3		

**CROSS SECTIONS
STA. 1127+25.00 TO STA. 1127+50.00**

HAS-22-21.26

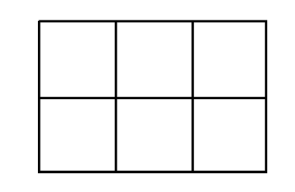
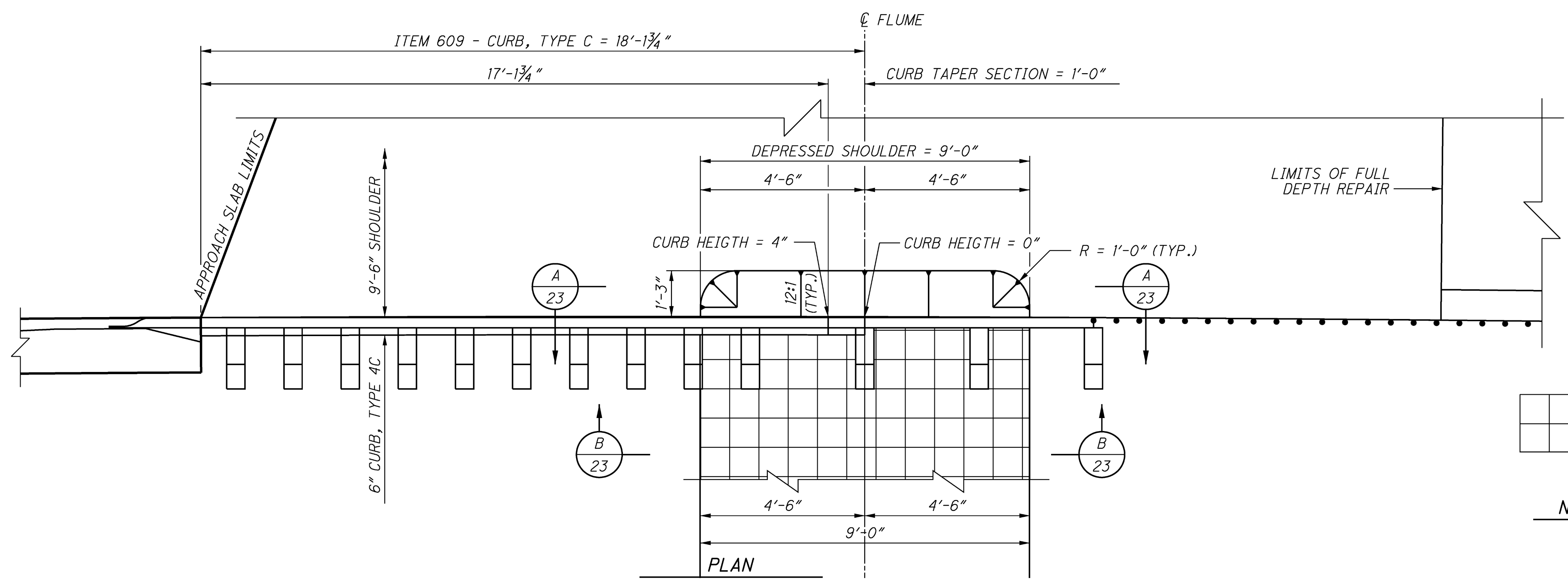
21
52

SHEET TOTALS CARRIED TO SHEET NO. 12

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81 SHEET TOTAL

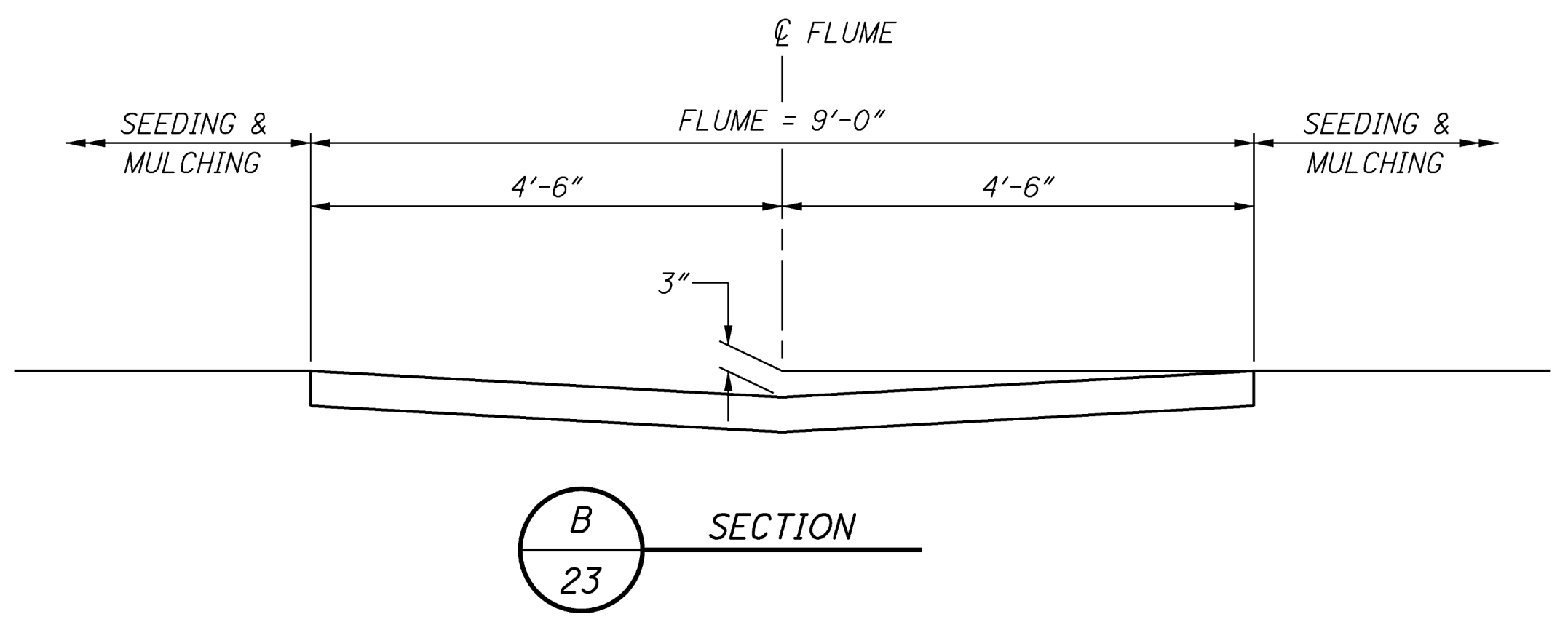
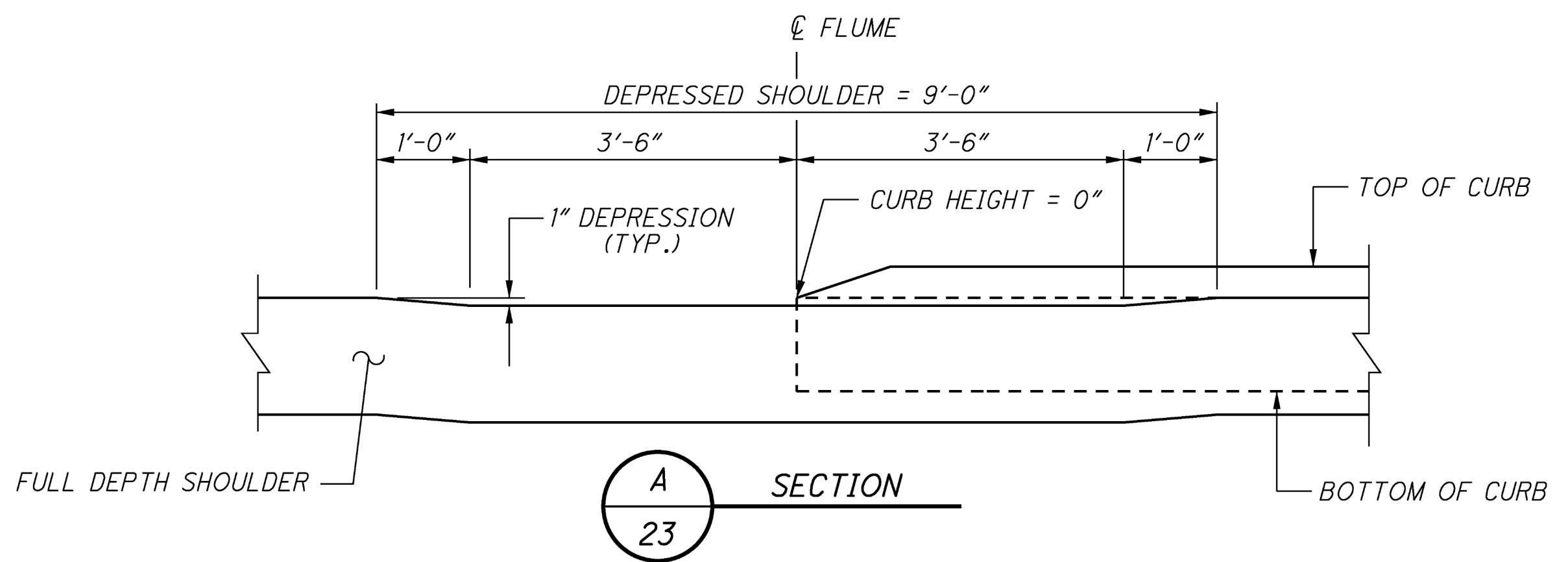
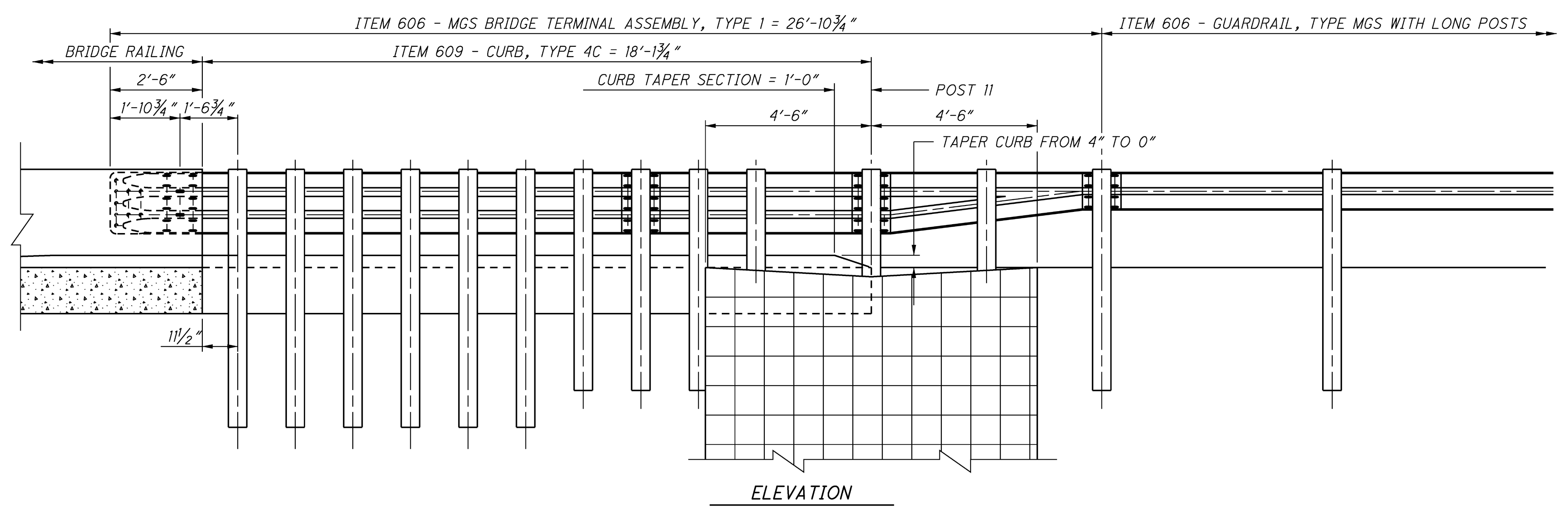
SUPERELEVATION												
LEFT SIDE					PROFILE CONTROL		RIGHT SIDE					REMARKS
EDGE ELEVATION	TRANSITION RATE	ELEVATION CORRECTION	CROSS SLOPE	WIDTH	STATION	PROFILE GRADE	WIDTH	CROSS SLOPE	ELEVATION CORRECTION	TRANSITION RATE	EDGE ELEVATION	
1150.97	2042	-1.00	-0.080	12.49	1122+50.00	1151.97	11.18	0.051	0.57	222	1152.54	MATCH EX.
1151.22		-0.99	-0.081	12.24	1122+75.00	1152.21	11.59	0.060	0.70		1152.91	FULL SUPER (LT)
1151.36		-0.98	-0.081	12.12	1122+88.19	1152.34	11.81	0.065	0.77		1153.11	S.C.
1151.48		-0.97	-0.081	12	1123+00.00	1152.45	12	0.070	0.84		1153.29	
1151.72		-0.97	-0.081	12	1123+25.00	1152.69	12	0.079	0.95		1153.64	
1151.77		-0.97	-0.081	12	1123+30.00	1152.74	12	0.081	0.97		1153.71	FULL SUPER (RT)
1151.81		-0.97	-0.081	12	1123+34.42	1152.78	12	0.081	0.97		1153.75	APPROACH SLAB
1151.96		-0.97	-0.081	12	1123+50.00	1152.93	12	0.081	0.97		1153.90	
1152.05		-0.97	-0.081	12	1123+59.08	1153.02	12	0.081	0.97		1153.99	
BRIDGE LIMITS												
1153.46		-0.97	-0.081	12	1125+05.58	1154.43	12	0.081	0.97		1155.40	APPROACH SLAB
1153.64		-0.97	-0.081	12	1125+25.00	1154.61	12	0.081	0.97		1155.58	
1153.69		-0.97	-0.081	12	1125+30.23	1154.66	12	0.081	0.97		1155.63	
1153.88		-0.97	-0.081	12	1125+50.00	1154.85	12	0.081	0.97		1155.82	
1154.12		-0.97	-0.081	12	1125+75.00	1155.09	12	0.081	0.97	299	1156.06	TRANSITION TO MATCH EX. (RT)
1154.36		-0.97	-0.081	12	1126+00.00	1155.33	12	0.074	0.89		1156.22	
1154.47		-0.98	-0.081	12.04	1126+11.52	1155.45	11.87	0.070	0.83		1156.28	C.S.
1154.59	256	-0.98	-0.081	12.10	1126+25.00	1155.57	11.71	0.066	0.77		1156.34	TRANSITION TO MATCH EX. (LT)
1154.92		-0.89	-0.073	12.19	1126+50.00	1155.81	11.42	0.059	0.67		1156.48	MATCH EX.



ITEM 601 - TIED CONCRETE BLOCK MAT, TYPE 2

NOTES

1. FOR DETAILS NOT SHOWN, SEE SCD MGS 3.1 & DM-4.1.
2. 20 SQ. YD. OF ITEM 659 - SEEDING & MULCHING HAS BEEN CARRIED TO THE EARTHWORK SUMMARY TABLE TO ACCOUNT FOR CONSTRUCTION OF THE TIED CONCRETE MAT.



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RAILROAD CURVE DATA
 P.I. Sta. 9745+50.64
 $\Delta = 9^\circ 59' 58''$ (RT)
 $D_c = 2^\circ 00' 00''$
 $R = 2,864.94'$
 $T = 250.64'$
 $L = 500.00'$
 $E = 10.94'$
 $C = 499.37'$
 C.B. = S $13^\circ 57' 16''$ E

U.S. 22 CURVE DATA
 P.I. Sta. 1124+53.30 $k = 199.95'$
 $\Delta = 18^\circ 05' 00''$ (LT) $p = 2.91'$
 $D_c = 2^\circ 30' 00''$ $\Delta c = 8^\circ 05' 00''$ (LT)
 $R = 2,291.83'$ $L_c = 323.33'$
 $L_s = 400.00'$ $E = 31.78'$
 $\theta_s = 5^\circ 00' 00''$ $C = 323.06'$
 $LT = 266.77'$ $C1 = C2 = 399.86'$
 $ST = 133.43'$ $C.B.1 = N 60^\circ 00' 16''$ E
 $x = 399.70'$ $C.B. = N 52^\circ 37' 46''$ E
 $y = 11.63'$ $C.B.2 = S 45^\circ 15' 16''$ W

BENCHMARK DATA

BM #1 STA. 1123+25.88 ELEV. 1152.76 OFFSET 58.17' RT
 BM #2 STA. 1126+40.57 ELEV. 1153.26 OFFSET 6.53' RT

FOR ADDITIONAL BENCHMARK INFORMATION. SEE ROADWAY PLAN SHEET 13/52

NOTES

EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.

DESIGN TRAFFIC:
 2015 ADT = 6200 2015 ADTT = 1302
 2035 ADT = 8000 2035 ADTT = 1680
 DIRECTIONAL DISTRIBUTION = 53%

LEGEND

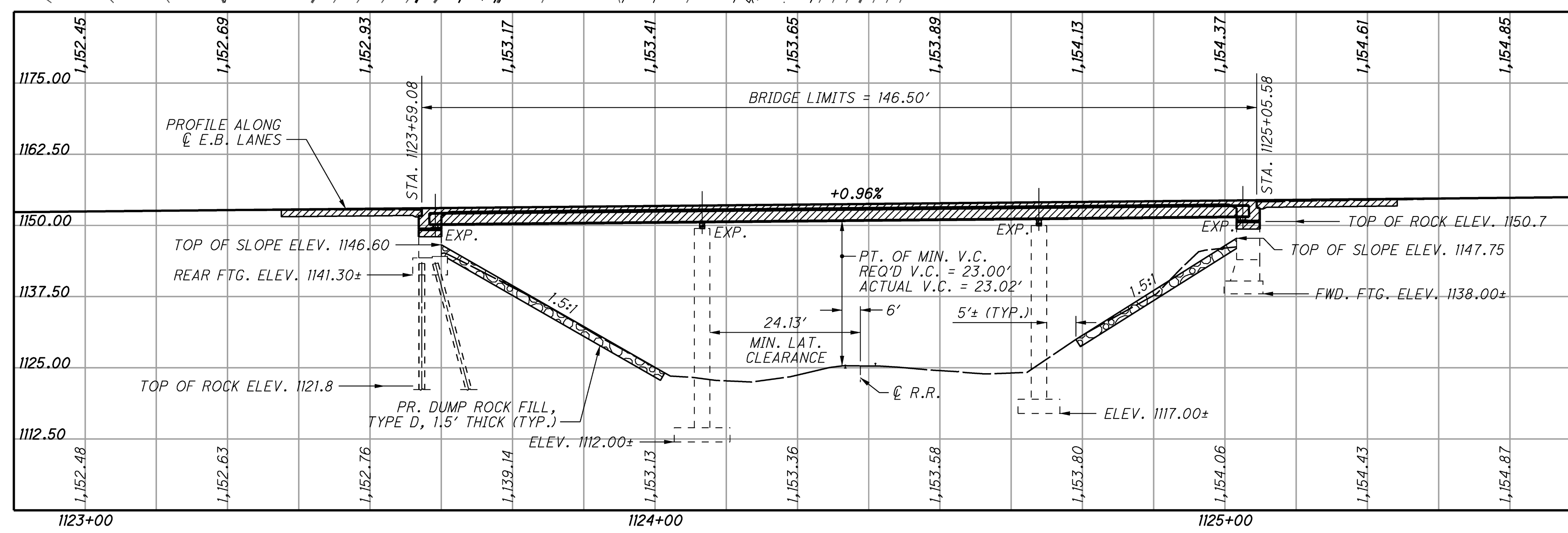
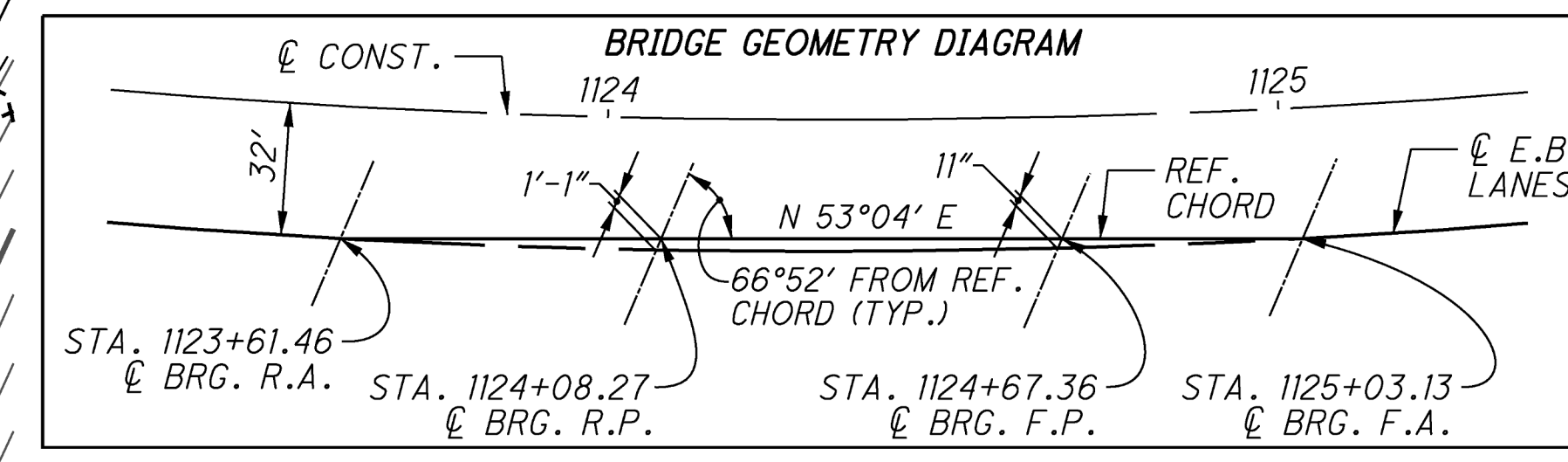
- ITEM 601 - DUMPED ROCK, TYPE D
- ITEM 601 - TIED CONCRETE BLOCK MAT, TYPE 2
- WETLAND AREA (TO BE AVOIDED)
- 23.00' REQUIRED MINIMUM VERTICAL CLEARANCE
23.02' ACTUAL MINIMUM VERTICAL CLEARANCE
- ⊙ SOIL BORING LOCATION

EXISTING STRUCTURE

TYPE: CONTINUOUS STEEL BEAM WITH REINFORCED CONCRETE DECK AND SUBSTRUCTURE WITH ONE SUBSTRUCTURE UNIT ON PILING AND ONE ON SPREAD FOOTING.
 SPANS: 47'-10 1/2"±, 59'-10 1/8"±, 35'-10 1/8"± c/c BRGS. (ALONG REF. CHORD)
 ROADWAY: 41'-8"± f/f SAFETY CURB
 LOADING: HS-20
 SKEW: 23°-08' L.F.
 APPROACH SLABS: AS-1-54 (25'-0"± LONG)
 ALIGNMENT: 2°-30' CURVE LEFT
 SUPERELEVATION: 0.081 FT./FT.
 STRUCTURAL FILE NUMBER: 3401200
 DATE BUILT: 1962
 EXISTING WEARING SURFACE: 1 1/4"± LATEX MODIFIED CONCRETE
 CONDITION: REMOVE SUPERSTRUCTURE, REHABILITATE ABUTMENTS

PROPOSED STRUCTURE

TYPE: NEW SEMI-INTEGRAL THREE-SPAN CONTINUOUS A709 GRADE 50W STEEL BEAMS WITH COMPOSITE REINFORCED CONCRETE DECK ON EXISTING REINFORCED CONCRETE SUBSTRUCTURE UNITS.
 SPANS: 47'-10 1/2"±, 59'-10 1/8"±, 35'-10 1/8"± c/c BRGS. (ALONG REF. CHORD)
 ROADWAY: 43'-0" T/T PARAPET
 LOADING: HS-20 CASE II AND ALTERNATE MILITARY LOADING (FWS=60PSF)
 SKEW: 23°-08'± L.F.
 APPROACH SLABS: 25'-0" LONG (AS-1-81)
 ALIGNMENT: 2°-30' CURVE LEFT
 SUPERELEVATION: 0.081 FT./FT.
 WEARING SURFACE: 1" MONOLITHIC CONCRETE
 COORDINATES: LATITUDE 40° 19' 00"
 LONGITUDE 80° 56' 40"



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REFERENCES

REFERENCE SHALL BE MADE TO STANDARD DRAWINGS:

AS-1-81:	REVISED	1/18/2013
GSD-1-96:	REVISED	7/19/2002
PCB-91	REVISED	1/18/2013
SBR-1-99	REVISED	7/19/2002
SICD-1-96	REVISED	7/19/2002

DESIGN SPECIFICATIONS

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2002, INCLUDING THE INTERIM SPECIFICATIONS AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING

HS20, CASE II AND THE ALTERNATE MILITARY LOADING. FUTURE WEARING SURFACE (FWS) OF 60 LBS/SQ. FT.

DESIGN DATA

CLASS QC2, CONCRETE - COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE).

CLASS QC1, CONCRETE - COMPRESSIVE STRENGTH 4000 PSI (SUBSTRUCTURE).

REINFORCING STEEL - ASTM A615 OR A996 GRADE 60, MINIMUM YIELD STRENGTH 60,000 PSI.

STRUCTURAL STEEL - ASTM A709 GRADE 50W, MINIMUM YIELD STRENGTH 50,000 PSI.

DECK PROTECTION METHOD:

EPOXY COATED REINFORCING STEEL, 2½" CONCRETE COVER.

MONOLITHIC WEARING SURFACE:

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1 INCH THICK.

EXISTING STRUCTURE VERIFICATION

DETAILS AND DIMENSIONS SHOWN ON THESE PLANS PERTAINING TO THE EXISTING STRUCTURE HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND FROM FIELD OBSERVATIONS AND MEASUREMENTS. CONSEQUENTLY, THEY ARE INDICATIVE OF THE EXISTING STRUCTURE AND THE PROPOSED WORK, BUT THEY SHALL BE CONSIDERED TENTATIVE AND APPROXIMATE. THE CONTRACTOR IS REFERRED TO CMS SECTIONS 102.05, 105.02 AND 513.04.

BASE CONTRACT BID PRICES UPON A RECOGNITION OF THE UNCERTAINTIES DESCRIBED ABOVE AND UPON A PRE-BID EXAMINATION OF THE EXISTING STRUCTURE. HOWEVER, THE DEPARTMENT WILL PAY FOR ALL PROJECT WORK BASED UPON ACTUAL DETAILS AND DIMENSIONS THAT HAVE BEEN VERIFIED IN THE FIELD.

PROPOSED WORK

1. REMOVE EXISTING DECK, APPROACH SLABS, WING WALLS, BEAMS, BEARINGS, AND PORTIONS OF BREAST WALLS.
2. DRIVE PILING, AND BUILD FOOTINGS FOR WING WALL FOUNDATIONS.
3. CONSTRUCT NEW BREAST WALLS AND WING WALLS.
4. REPLACE POROUS BACKFILL AND PLACE R.C.P.
5. INSTALL NEW ELASTOMERIC BEARINGS AND ERECT NEW STEEL BEAMS.
6. CONSTRUCT ABUTMENT DIAPHRAGMS.
7. CONSTRUCT CONCRETE APPROACH SLABS, DECK, AND PARAPET.
8. SEAL CONCRETE SURFACES.

ITEM 202 - PORTIONS OF STRUCTURE REMOVED, OVER 20 FT SPAN, AS PER PLAN

ALL APPLICABLE PROVISIONS OF ITEM 202 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS SHALL BE OBSERVED IN THE PERFORMANCE OF THIS ITEM EXCEPT AS MODIFIED HEREIN.

COMPLETELY REMOVE THE EXISTING SUPERSTRUCTURE AND REMOVE PORTIONS OF THE ABUTMENTS AS SHOWN IN THE REMOVAL DETAILS ON SHEETS 5[29] & 6[29].

DETAILS:

THIS WORK SHALL INCLUDE THE COMPLETE REMOVAL OF THE EXISTING CONCRETE DECK INCLUDING PARAPETS, RAILINGS, DECK JOINTS, BEARINGS, STRUCTURAL STEEL (BEAMS, CROSS FRAMES, SCUPPERS, ETC.), AND OTHER APPURTANCES. IT SHALL ALSO INCLUDE THE REMOVAL OF THE ABUTMENT BACK WALLS, PORTIONS OF THE WING WALLS, PORTIONS OF THE ABUTMENT BREAST WALL, POROUS BACKFILL, AND OTHER APPURTANCES AS SHOWN IN THE REMOVAL PLANS. ITEMS TO BE REMOVED INCLUDE ALL EXISTING MATERIALS BEING REPLACED BY NEW CONSTRUCTION AND MISCELLANEOUS ITEMS THAT ARE NOT SHOWN TO BE INCORPORATED INTO THE FINAL CONSTRUCTION AND ARE DIRECTED TO BE REMOVED BY THE ENGINEER. CARE SHALL BE TAKEN DURING THE REMOVALS TO PROTECT PORTIONS OF SUCH SYSTEMS THAT ARE TO BE SALVAGED AND INCORPORATED INTO THE PROPOSED STRUCTURE.

IN THIS RESPECT THE USE OF EXPLOSIVES, HEADACHE BALLS AND/OR HOE-RAM TYPE EQUIPMENT IS PROHIBITED. THE METHOD OF REMOVAL AND THE WEIGHT OF THE HAMMER SHALL BE APPROVED BY THE ENGINEER. PERFORM ALL WORK IN A MANNER THAT WILL NOT CUT, ELONGATE OR DAMAGE THE EXISTING REINFORCING STEEL TO BE PRESERVED IN THE NEW CONSTRUCTION. PNEUMATIC HAMMERS SHALL NOT BE PLACED IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE REBUILT STRUCTURE. THE CONTRACTOR SHALL SUBMIT REMOVAL PLANS IN ACCORDANCE WITH CMS 501.05.

PROTECTION OF TRAFFIC:

PRIOR TO ANY DEMOLITION OF THE EXISTING STRUCTURE, THE CONTRACTOR SHALL SUBMIT PLANS FOR THE PROTECTION OF TRAFFIC IN ACCORDANCE WITH CMS 501.05.

REMOVAL METHODS:

CONCRETE MAY BE REMOVED BY CUTTING AND BY MEANS OF HAND OPERATED PNEUMATIC HAMMER EMPLOYING POINTED OR BLUNTED CHISEL TYPE TOOLS.

ITEM 202 - PORTIONS OF STRUCTURE REMOVED, OVER 20 FT SPAN, AS PER PLAN (CONTINUED...)

CUT LINE CONSTRUCTION JOINT PREPARATION: SAW CUT BOUNDARIES OF PROPOSED CONCRETE REMOVALS 1 INCH DEEP. REMOVE CONCRETE TO A ROUGH SURFACE. LEAVE THE EXISTING REINFORCING STEEL, IF REQUIRED IN THE PLANS, IN PLACE. INSTALL DOWEL BARS IF SPECIFIED. PRIOR TO CONCRETE PLACEMENT ABRASIVELY CLEAN JOINT SURFACES AND EXISTING EXPOSED REINFORCING TO REMOVE LOOSE AND DISINTEGRATED CONCRETE AND LOOSE RUST. THOROUGHLY CLEAN THE JOINT SURFACE AND EXPOSED REINFORCEMENT OF ALL DIRT, DUST, RUST, OR OTHER FOREIGN MATERIAL BY THE USE OF WATER, AIR UNDER PRESSURE, OR OTHER METHODS THAT PRODUCE SATISFACTORY RESULTS. EXISTING REINFORCING STEEL DOES NOT NEED TO HAVE A BRIGHT STEEL FINISH, BUT REMOVE ALL PACK AND LOOSE RUST. THOROUGHLY DRENCH EXISTING CONCRETE SURFACES WITH CLEAN WATER AND ALLOW TO DRY TO A DAMP CONDITION BEFORE PLACING CONCRETE.

SUBSTRUCTURE CONCRETE REMOVAL:

REMOVE CONCRETE BY MEANS OF APPROVED PNEUMATIC HAMMERS EMPLOYING POINTED AND BLUNT CHISEL TOOLS. HYDRAULIC HOE-RAM TYPE HAMMERS WILL NOT BE PERMITTED. THE WEIGHT OF HAMMER SHALL NOT BE MORE THAN 35 POUNDS FOR REMOVAL WITHIN 18 INCHES OF PORTIONS TO BE PRESERVED. OUTSIDE THE 18 INCH LIMIT, THE CONTRACTOR MAY USE HAMMERS NOT EXCEEDING 90 POUNDS UPON THE APPROVAL OF THE ENGINEER. DO NOT PLACE PNEUMATIC HAMMERS IN DIRECT CONTACT WITH REINFORCING STEEL THAT IS TO BE RETAINED IN THE REBUILT STRUCTURE.

PAYMENT:

THE DEPARTMENT WILL MEASURE THE QUANTITY OF REMOVALS ON A LUMP SUM BASIS. THE DEPARTMENT WILL PAY FOR THE ACCEPTED QUANTITIES OF REMOVALS AT THE CONTRACT PRICE FOR ITEM 202, PORTIONS OF STRUCTURE REMOVED, AS PER PLAN.

INSPECTION OF BRIDGE FOR BATS

PRIOR TO ANY DEMOLITION/REMOVAL OF THE EXISTING BRIDGE, THE CONTRACTOR SHALL CAREFULLY EXAMINE THE UNDERSIDE OF THE STRUCTURE FOR THE PRESENCE OF BATS. IF ANY BATS ARE FOUND, THE O.D.O.T. DISTRICT II ENVIRONMENTAL COORDINATOR SHOULD BE CONTACTED AT 330-339-6633 BEFORE COMMENCING WITH THE BRIDGE'S DEMOLITION.

ASBESTOS NOTIFICATION

AN ASBESTOS SURVEY OF THE BRIDGE STRUCTURE SCHEDULED FOR REHABILITATION WAS CONDUCTED BY A CERTIFIED ASBESTOS HAZARD EVALUATION SPECIALIST. THE SURVEY DETERMINED THAT NO ASBESTOS IS PRESENT ON THE BRIDGE STRUCTURE.

A COPY OF THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) NOTIFICATION OF DEMOLITION AND RENOVATION FORMS, PARTIALLY COMPLETED AND SIGNED BY THE BRIDGE OWNER, WILL BE PROVIDED TO THE SUCCESSFUL BIDDER. THE CONTRACTOR SHALL COMPLETE THE FORM AND SUBMIT IT TO THE ADDRESS BELOW AT LEAST TEN (10) WORKING DAYS PRIOR TO THE START OF ANY DEMOLITION AND/OR RENOVATION.

MR. STEVE LOWRY
OHIO EPA, SEDO
2195 FRONT STREET
LOGAN, OHIO 43138

ASBESTOS NOTIFICATION (CONTINUED)

THE CONTRACTOR SHALL PROVIDE A COPY OF THE COMPLETED FORM TO THE ENGINEER. INFORMATION REQUIRED ON THE FORM WILL INCLUDE: 1) THE CONTRACTORS NAME AND ADDRESS, 2) THE SCHEDULED DATES FOR THE START AND COMPLETION OF THE BRIDGE REMOVAL AND 3) A DESCRIPTION OF THE PLANNED DEMOLITION WORK AND THE METHOD(S) TO BE USED. COPIES OF THE OEPA FORM AND BRIDGE INSPECTION REPORT ARE AVAILABLE FOR REVIEW AT THE ODOT DISTRICT II OFFICE, 2201 REISER AVENUE, NEW PHILADELPHIA, OHIO 44663.

BASIS FOR PAYMENT THE CONTRACTOR SHALL FURNISH ALL FEES, LABOR, AND MATERIAL NECESSARY TO COMPLETE AND SUBMIT THE OEPA NOTIFICATION FORM. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN ITEM 202 PORTIONS OF STRUCTURE REMOVED, AS PER PLAN.

ITEM 503 - COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN

THE DESIGN OF THE TEMPORARY EXCAVATION BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER, AS PER THE REQUIREMENTS OF CMS 501.05. UPON COMPLETION OF THE PROJECT, THE TEMPORARY SHORING SHALL BE REMOVED AT LEAST 2 FEET BELOW THE FINISHED GRADE.

THE DEPARTMENT WILL PAY FOR THE TEMPORARY SUPPORT OF EXCAVATION AT THE CONTRACT LUMP SUM PRICE FOR ITEM 503, COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN.

ITEM 511, CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK, AS PER PLAN

THE BRIDGE DECK GROOVING SHALL BE PARALLEL TO THE CENTERLINE OF CONSTRUCTION OF U.S. 22. ALL OTHER PROVISIONS OF 499 AND 511 STILL APPLY.

ITEM 518 - STRUCTURE DRAINAGE, MISC.: PLUG EXISTING 4"± WEEP HOLES

PLUG THE WEEP HOLES IN THE EXISTING BREASTWALL WITH MATERIAL CONFORMING TO CMS 510.02.

PAYMENT SHALL INCLUDE ALL LABOR, EQUIPMENT, MATERIALS AND INCIDENTALS NECESSARY TO COMPLETE THE ABOVE WORK.

DECK PLACEMENT DESIGN ASSUMPTIONS

THE FOLLOWING ASSUMPTIONS OF THE CONSTRUCTION MEANS AND METHODS WERE MADE FOR THE ANALYSIS AND DESIGN OF THE SUPERSTRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF THE FALSEWORK SUPPORT SYSTEM WITHIN THESE PARAMETERS AND WILL ASSUME RESPONSIBILITY FOR SUPERSTRUCTURE ANALYSIS FOR DEVIATION FROM THESE DESIGN ASSUMPTIONS.

AN EIGHT WHEEL FINISHING MACHINE WITH A MAXIMUM WHEEL LOAD OF 1.188 KIPS FOR A TOTAL MACHINE LOAD OF 9.5 KIPS.

A MINIMUM OUT-TO-OUT WHEEL SPACING AT EACH END OF THE MACHINE OF 103".

A MAXIMUM SPACING OF OVERHANG FALSEWORK OF 48".

A MINIMUM DISTANCE FROM THE CENTERLINE OF THE FASCIA GIRDER TO THE FACE OF THE SAFETY HANDRAIL OF 65".

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DESIGN AGENCY
O.D.O.T. DISTRICT II
PLANNING & ENGINEERING

DATE
REVIEWED
STRUCTURE FILE NUMBER
340200

DRAWN
CCN
DESIGNED
CCN
CHECKED
RPT

STRUCTURE NOTES
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
PID No. 88904

2 / 29

25
52

PILES TO BEDROCK

PILES TO BEDROCK: DRIVE PILES TO REFUSAL ON BEDROCK. THE DEPARTMENT WILL CONSIDER REFUSAL TO BE OBTAINED BY PENETRATING WEAK BEDROCK FOR SEVERAL INCHES TO A MINIMUM RESISTANCE OF 20 BLOWS PER INCH OR BY CONTACTING STRONG BEDROCK AND THE PILE RECEIVING AT LEAST 20 BLOWS. SELECT THE HAMMER SIZE TO ACHIEVE THE REQUIRED DEPTH TO BEDROCK AND REFUSAL. INSTEAD OF DRIVING TO REFUSAL, THE CONTRACTOR MAY PERFORM DYNAMIC LOAD TESTING ACCORDING TO CMS 523 TO ESTABLISH A DRIVING CRITERIA FOR EACH PILE TYPE AND CAPACITY. ESTABLISH THE DRIVING CRITERIA TO ACHIEVE AN ULTIMATE BEARING VALUE GIVEN BELOW FOR THE PILES. PAYMENT FOR DYNAMIC LOAD TESTING PERFORMED AT THE CONTRACTOR'S OPTION IS INCLUDED IN THE UNIT PRICE PAY ITEM FOR PILES DRIVEN.

THE ULTIMATE BEARING VALUE IS 190 TONS PER PILE FOR THE 6 ABUTMENT PILES.

ABUTMENT PILES:

6 - HP12X53 PILES 35 FEET LONG, ORDER LENGTH

PILE SPLICES

PILE SPLICES: IN LIEU OF USING THE FULL PENETRATION BUTT WELDS SPECIFIED IN CMS 507.09 TO SPLICE STEEL H-PILES, THE CONTRACTOR MAY USE A MANUFACTURED H-PILE SPLICER. FURNISH SPLICERS FROM THE FOLLOWING MANUFACTURER:

ASSOCIATED PILE AND FITTING CORPORATION
8 WOOD HOLLOW RD. PLAZA 1
PARSIPPANY, NJ 07054

INSTALL AND WELD THE SPLICER TO THE PILE SECTIONS IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN ASSEMBLY PROCEDURE SUPPLIED TO THE ENGINEER BEFORE THE WELDING IS PERFORMED.

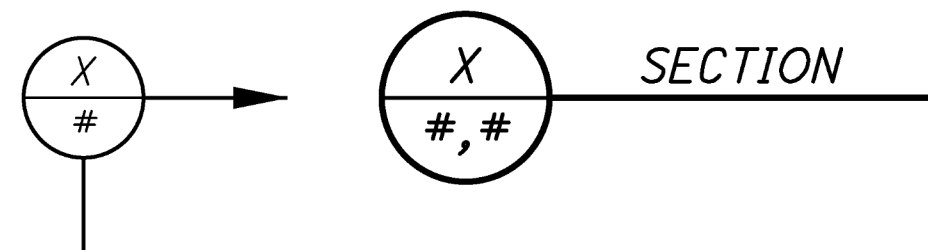
FOUNDATION BEARING PRESSURE

WINGWALL FOOTINGS, AS DESIGNED, PRODUCE A MAXIMUM BEARING PRESSURE OF 1.34 TONS PER SQUARE FOOT. THE ALLOWABLE BEARING PRESSURE IS 4 TONS PER SQUARE FOOT.

ABBREVIATIONS:

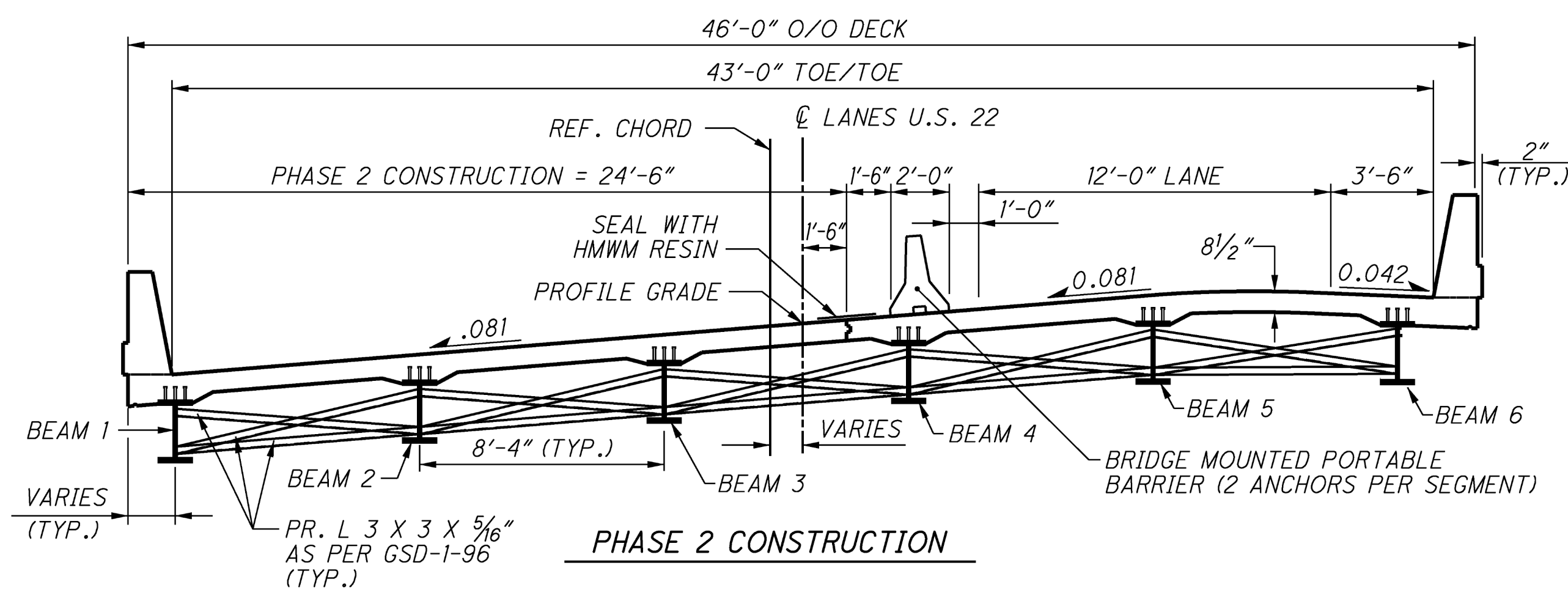
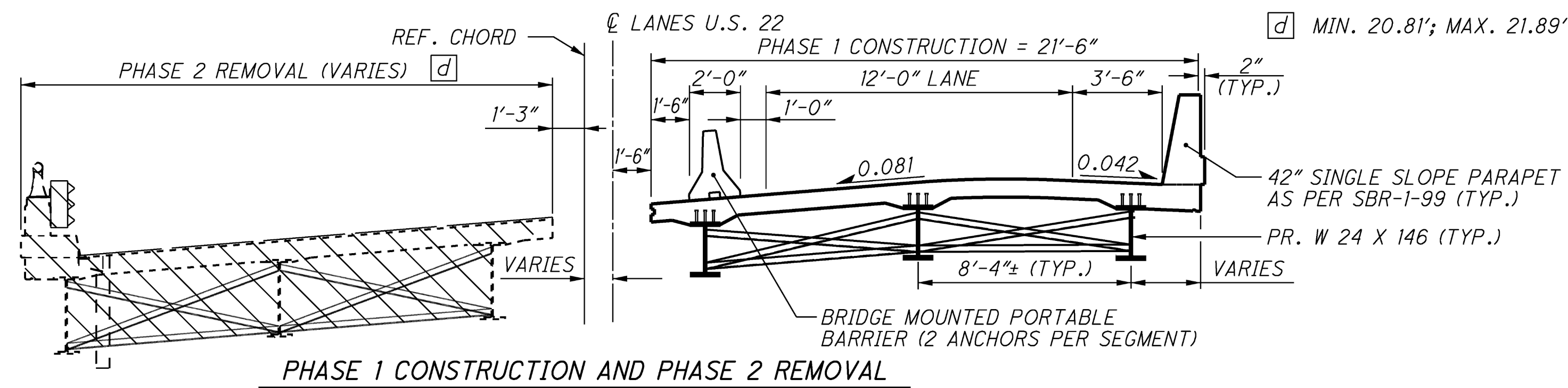
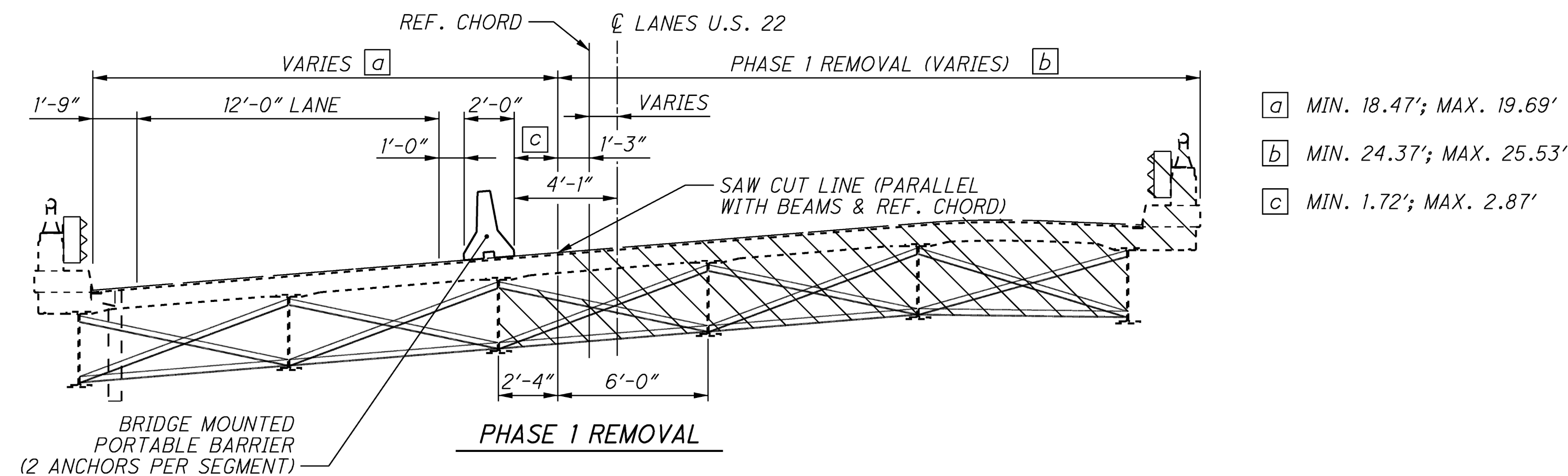
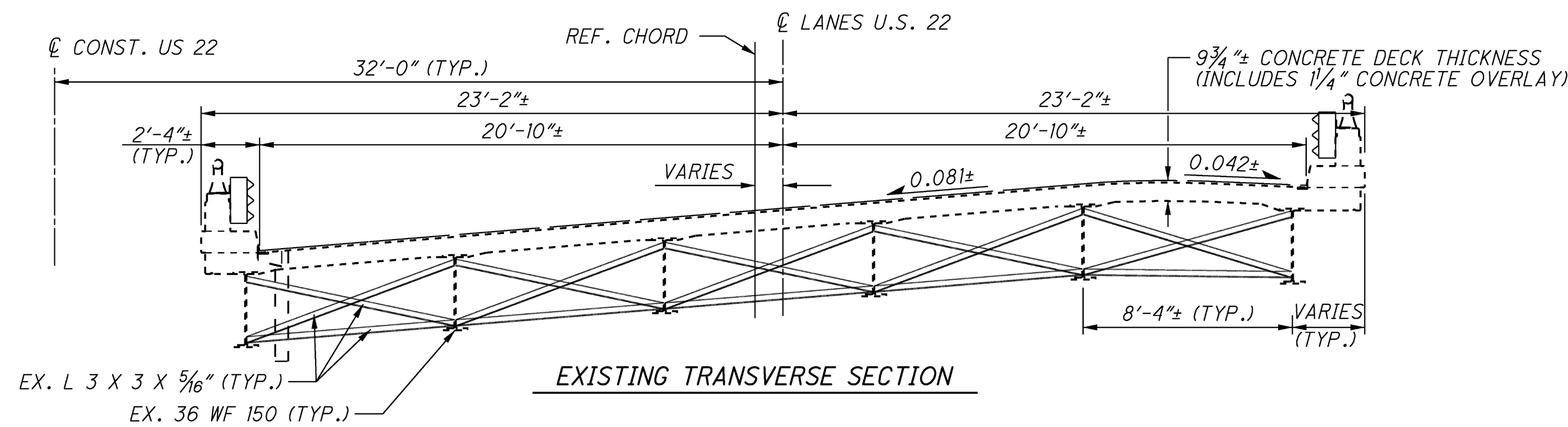
- BRG. - BEARING
- C.J. - CONSTRUCTION JOINT
- CONST. - CONSTRUCTION
- E.F. - EACH FACE
- EL. OR ELEV. - ELEVATION
- EX. - EXISTING
- EXP. - EXPANSION
- F.A. - FORWARD ABUTMENT
- F.F. - FAR FACE
- F.P. - FORWARD PIER
- FTG. - FOOTING
- FWD - FORWARD
- JT. - JOINT
- MAX. - MAXIMUM
- MIN. - MINIMUM
- N.F. - NEAR FACE
- N.P.C.P.P. - NON-PERFORATED CORRUGATED PLASTIC PIPE
- O.C.J. - OPTIONAL CONSTRUCTION JOINT
- O.H.W.M - ORDINARY HIGH WATER MARK
- P.C.P.P. - PERFORATED CORRUGATED PLASTIC PIPE
- PR. - PROPOSED
- R.A. - REAR ABUTMENT
- R.C.P. - ROCK CHANNEL PROTECTION
- REF. - REFERENCE
- R.R. - RAILROAD
- R.P. - REAR PIER
- STA. - STATION
- STD. DWG. OR SCD - STANDARD CONSTRUCTION DRAWING
- TYP. - TYPICAL

THE SYMBOLS BELOW DESIGNATE THE NAMES AND LOCATIONS OF THE SECTION DETAILS THROUGHOUT THE STRUCTURE PLANS. THE TOP LETTER DESIGNATES THE SECTION NAME. THE BOTTOM NUMBER(S) SHOW WHICH STRUCTURE SHEET NUMBER IS BEING CROSS REFERENCED.



DESIGNED CCN	CHECKED RPT	DRAWN CCN	REVIEWED	DATE	DESIGN AGENCY
		REVISED	STRUCTURE FILE NUMBER		O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING
STRUCTURE NOTES					
BRIDGE NO. HAS-22-2126 OVER WHEELING & LAKE ERIE RAILROAD					
HAS - 22 - 21.26 PID No. 88904					
3 / 29					
26 52					

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PHASE 1 REMOVAL

1. INSTALL AND MAINTAIN CONSTRUCTION SIGNS AND SIGNALS AS SHOWN ON SHEET 7 OF 52 AND AS PER SCD MT-96.11.
2. ERECT BRIDGE MOUNTED PORTABLE BARRIER (2 ANCHORS PER SEGMENT) ON THE LEFT PORTION OF THE EXISTING STRUCTURE AS SHOWN AND AS PER SCD PCB-91. REMOVE CONFLICTING EXISTING PAVEMENT MARKINGS, AND INSTALL WORK ZONE PAVEMENT MARKINGS AS SHOWN ON SHEET 7 OF 52.
3. USE SIGNALS TO MAINTAIN ONE-LANE, TWO-WAY TRAFFIC ON THE LEFT PORTION OF U.S. 22 AS PER THE DETAILS SHOWN ON SHEET 7 OF 52.
4. SAW CUT THE EXISTING DECK AS SHOWN.
5. REMOVE THE RIGHT PORTION OF THE EXISTING DECK, PARAPET, BEAMS, AND CROSSFRAMES AS SHOWN. CUT AND REMOVE THE RIGHT PORTION OF THE REAR AND FORWARD ABUTMENTS AND APPROACH SLABS. DRIVE SHEET PILING WHEN NECESSARY.

PHASE 1 CONSTRUCTION & PHASE 2 REMOVAL

6. CONSTRUCT THE PROPOSED BEAMS, CROSSFRAMES, DECK, PARAPET, ABUTMENTS, AND APPROACH SLABS FOR THE RIGHT PORTION OF THE BRIDGE. PLACE PAVEMENT UP THROUGH THE INTERMEDIATE COURSE ON THE RIGHT SIDE OF U.S. 22, AND CONSTRUCT THE RIGHT SIDE GUARDRAIL.
7. ERECT BRIDGE MOUNTED PORTABLE BARRIER (TWO ANCHORS PER SEGMENT) ON THE NEWLY CONSTRUCTED RIGHT PORTION OF THE DECK AS SHOWN AND AS PER SCD PCB-91. REMOVE CONFLICTING EXISTING PAVEMENT MARKINGS, AND INSTALL WORK ZONE PAVEMENT MARKINGS AS SHOWN ON SHEET 8 OF 52. PROVIDE ALL MAINTENANCE OF TRAFFIC DEVICES.
8. USE SIGNALS TO MAINTAIN ONE-LANE, TWO-WAY TRAFFIC ON THE RIGHT PORTION OF U.S. 22 AS PER THE DETAILS SHOWN ON SHEETS 7 AND 8 OF 52.
9. REMOVE THE REMAINING PORTION OF THE EXISTING STRUCTURE AS SHOWN. CUT AND REMOVE THE LEFT PORTION OF THE REAR AND FORWARD ABUTMENTS AND APPROACH SLABS. DRIVE SHEET PILING WHEN NECESSARY.

PHASE 2 CONSTRUCTION

10. PLACE THE PROPOSED BEAMS FOR THE LEFT SIDE OF THE DECK AND PLACE CROSSFRAMES BETWEEN BEAMS 1 AND 2, AND 2 AND 3. CONSTRUCT THE LEFT PORTION OF THE PROPOSED DECK, PARAPET, ABUTMENTS, AND APPROACH SLABS. PLACE PAVEMENT UP THROUGH THE INTERMEDIATE COURSE ON THE LEFT SIDE OF U.S. 22, AND CONSTRUCT THE LEFT SIDE GUARDRAIL.
11. PLACE THE CROSSFRAMES BETWEEN BEAMS 3 AND 4.
12. IN ONE CONTINUOUS OPERATION, REMOVE THE ANCHORED PORTABLE CONCRETE BARRIER, TRAFFIC SIGNALS, AND CONFLICTING PAVEMENT MARKINGS.
13. SAW CUT GROOVES INTO DECK SURFACE, FILL ANCHOR HOLES WITH 705.20 GROUT AND SEAL THE DECK AT THE CONSTRUCTION JOINT WITH HMWM RESIN MAINTAINING TRAFFIC AS PER MT-97.10
14. AS SHOWN ON THE ROADWAY SHEETS, PLACE PAVEMENT SURFACE COURSE FOR THE ENTIRE PROJECT UNDER FLAGGERS AS PER SCD MT-97.10.
16. OPEN ROAD TO TWO-WAY TRAFFIC.

LEGEND:

 PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN

DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING

DATE
REVIEWED
STRUCTURE FILE NUMBER
3401200

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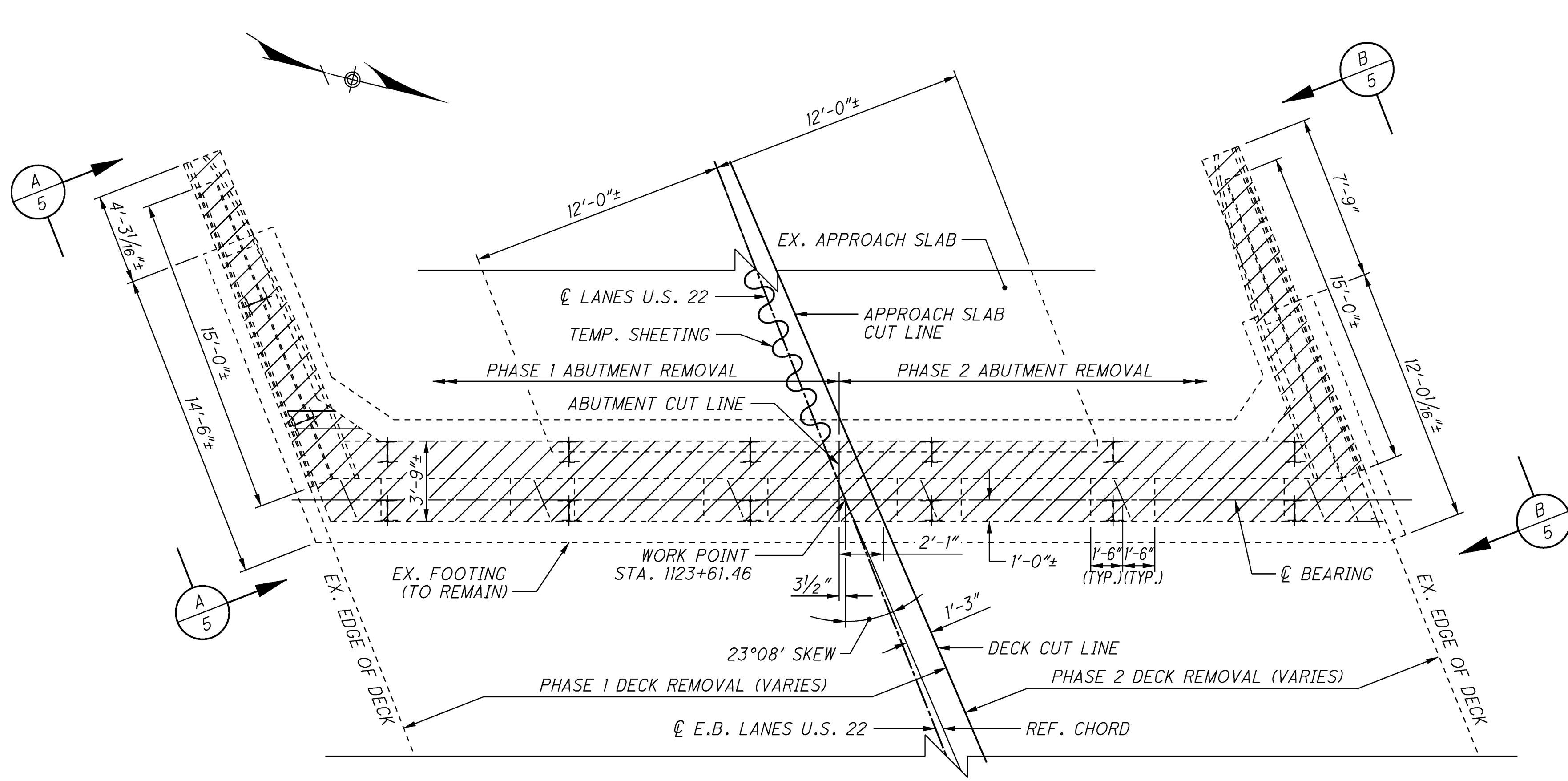
PHASE CONSTRUCTION DETAILS
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
PID No. 88904

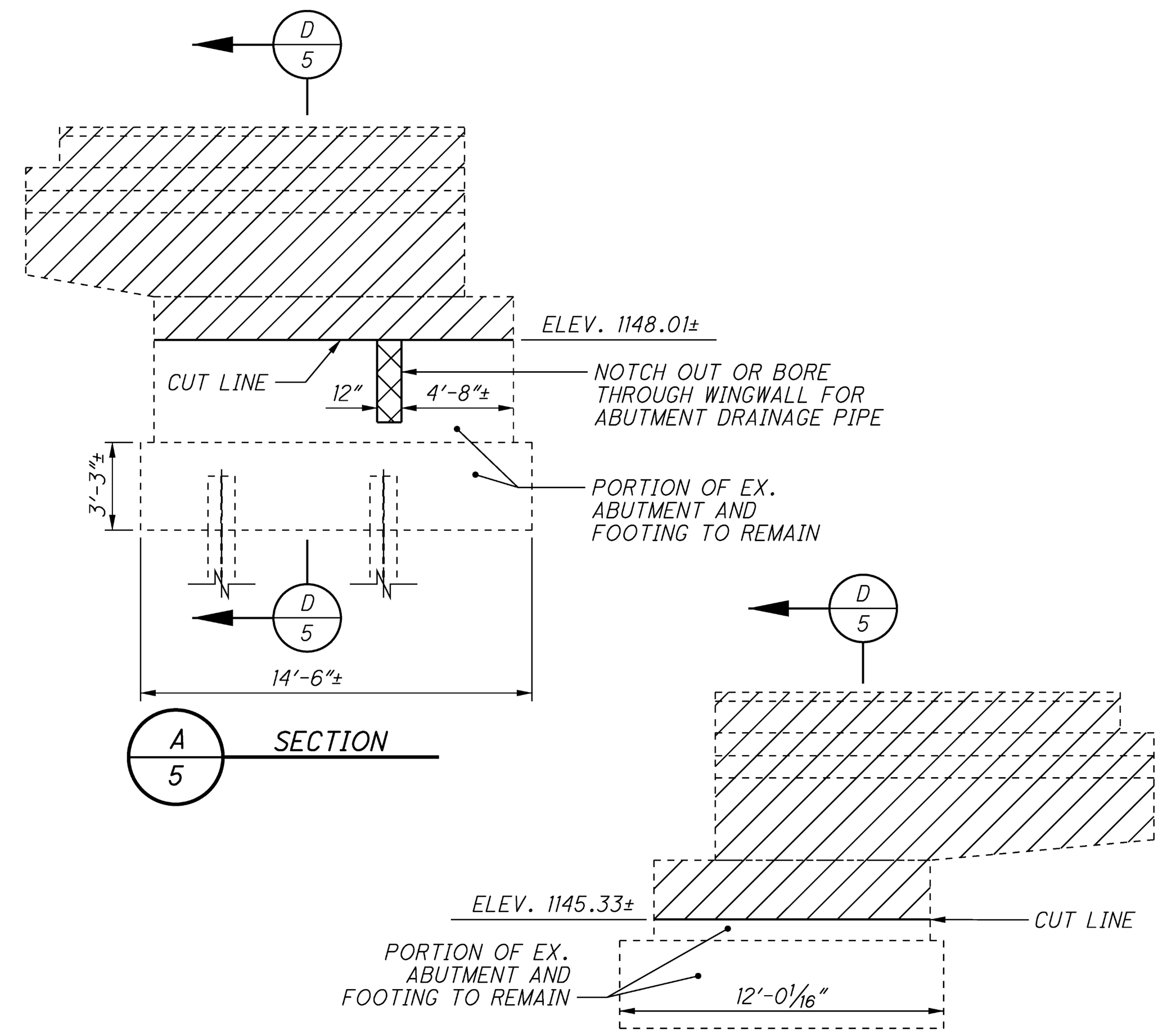
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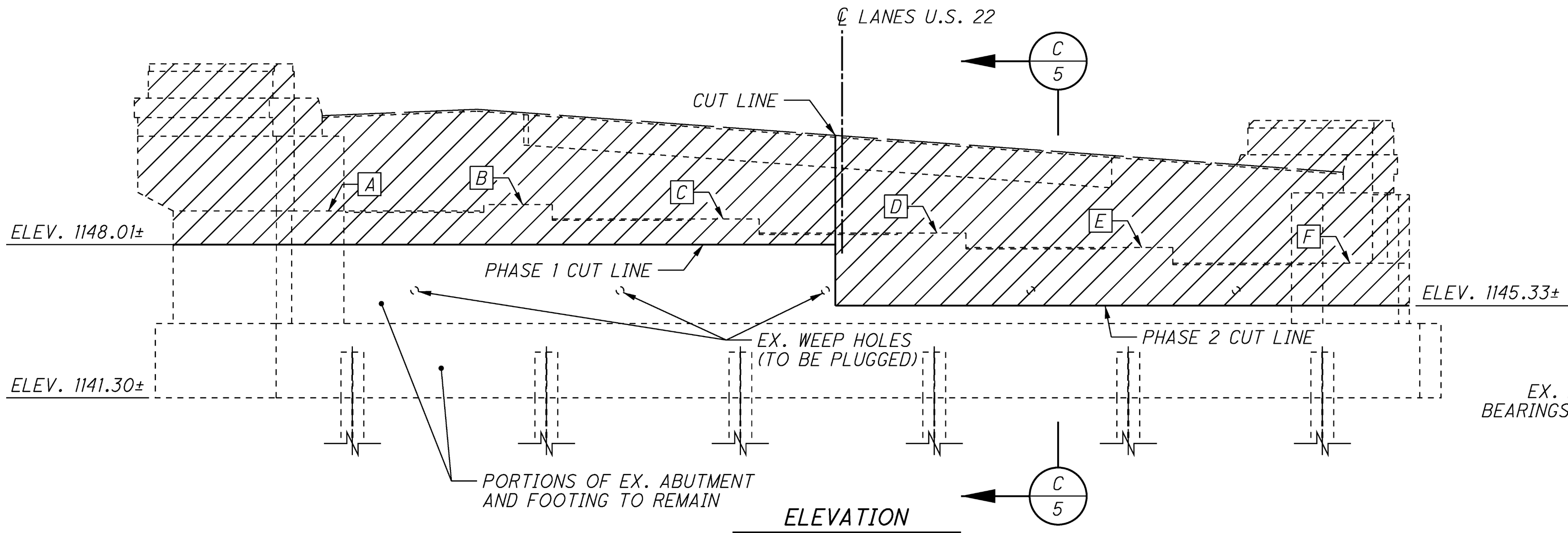


PLAN

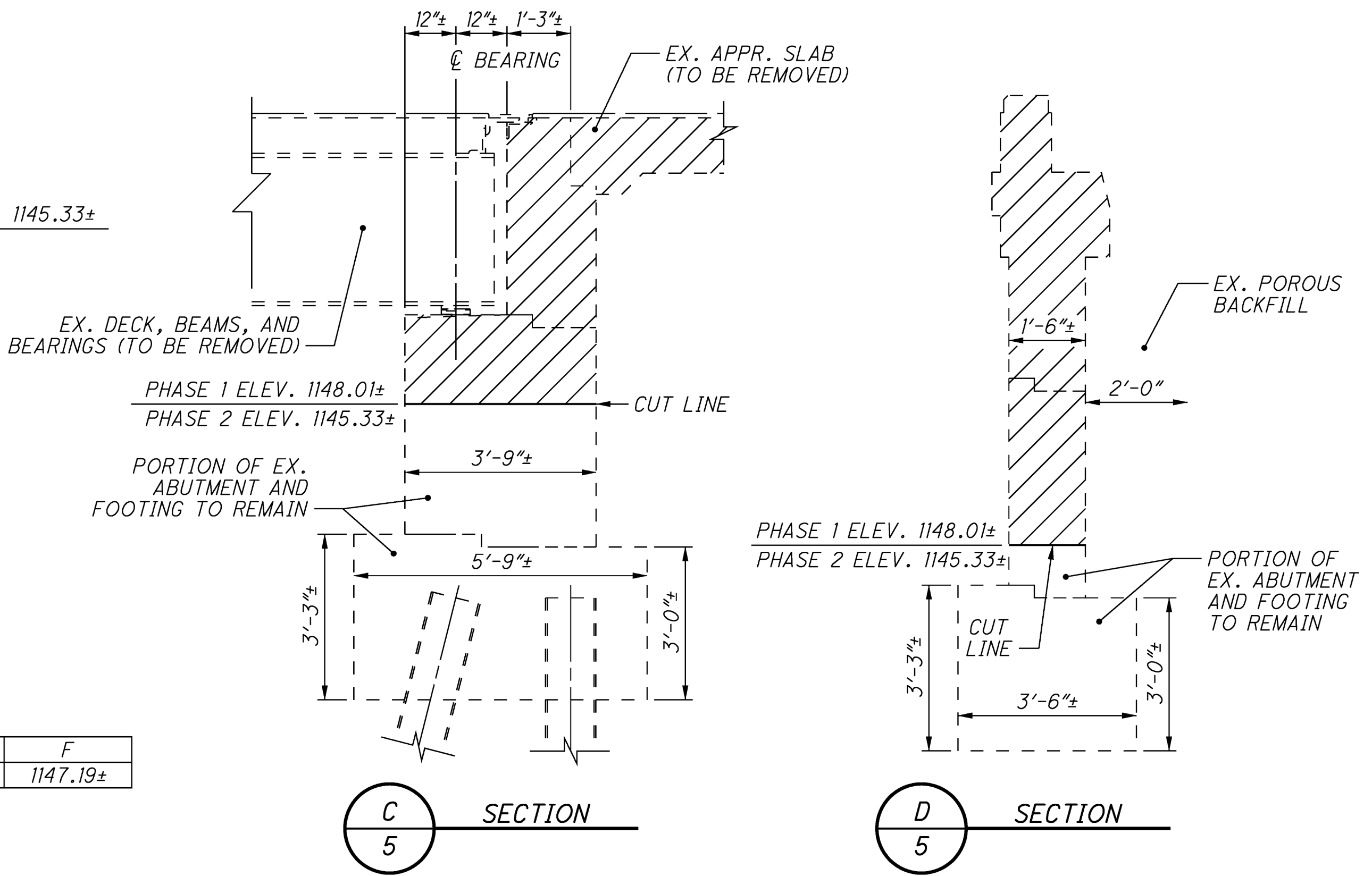


SECTION A-5

SECTION B-5



ELEVATION



SECTION C-5

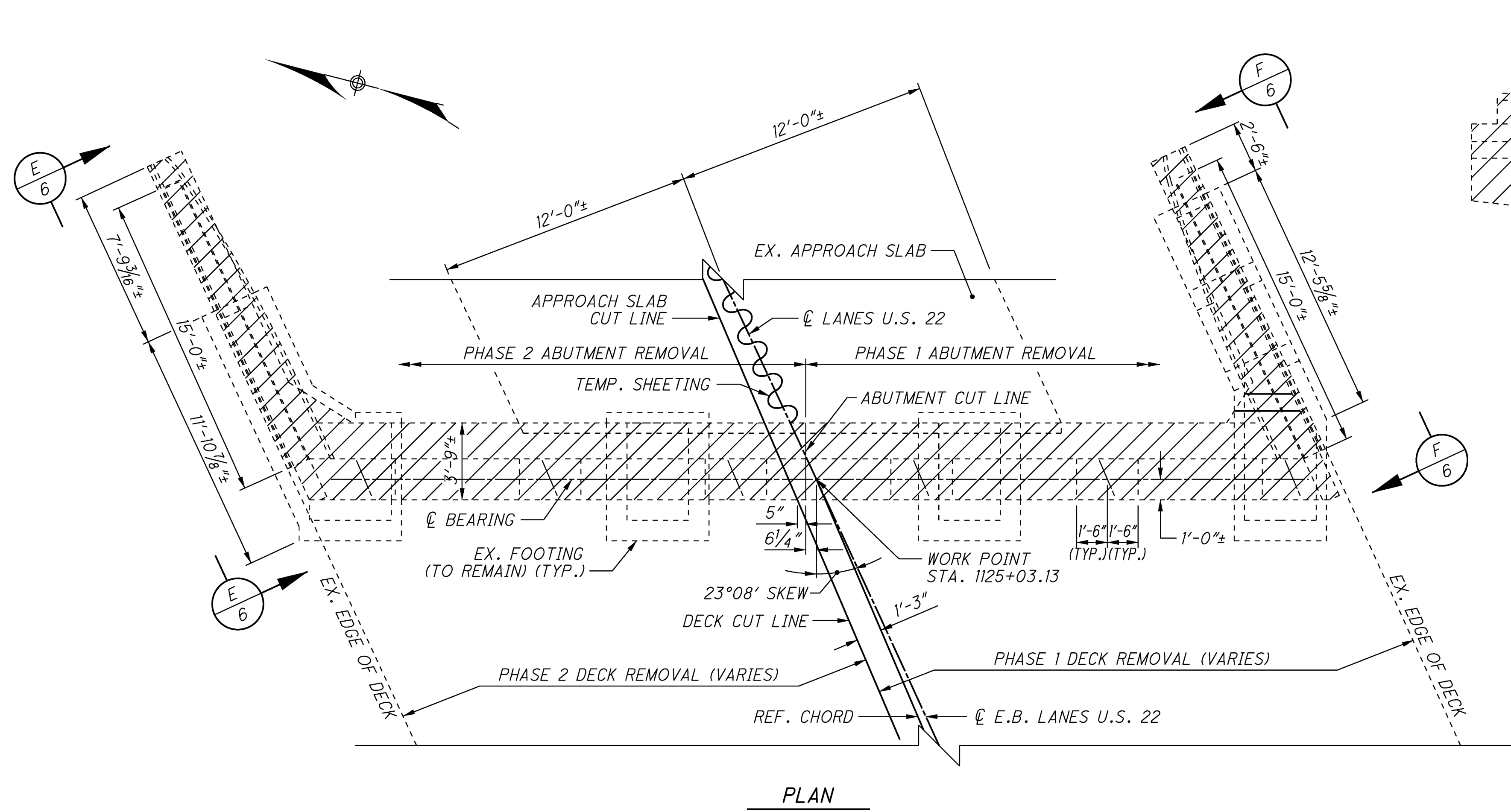
SECTION D-5

- PORTIONS OF STRUCTURE TO BE REMOVED
- PORTION OF WINGWALL REMOVED TO PLACE ABUTMENT DRAINAGE PIPE

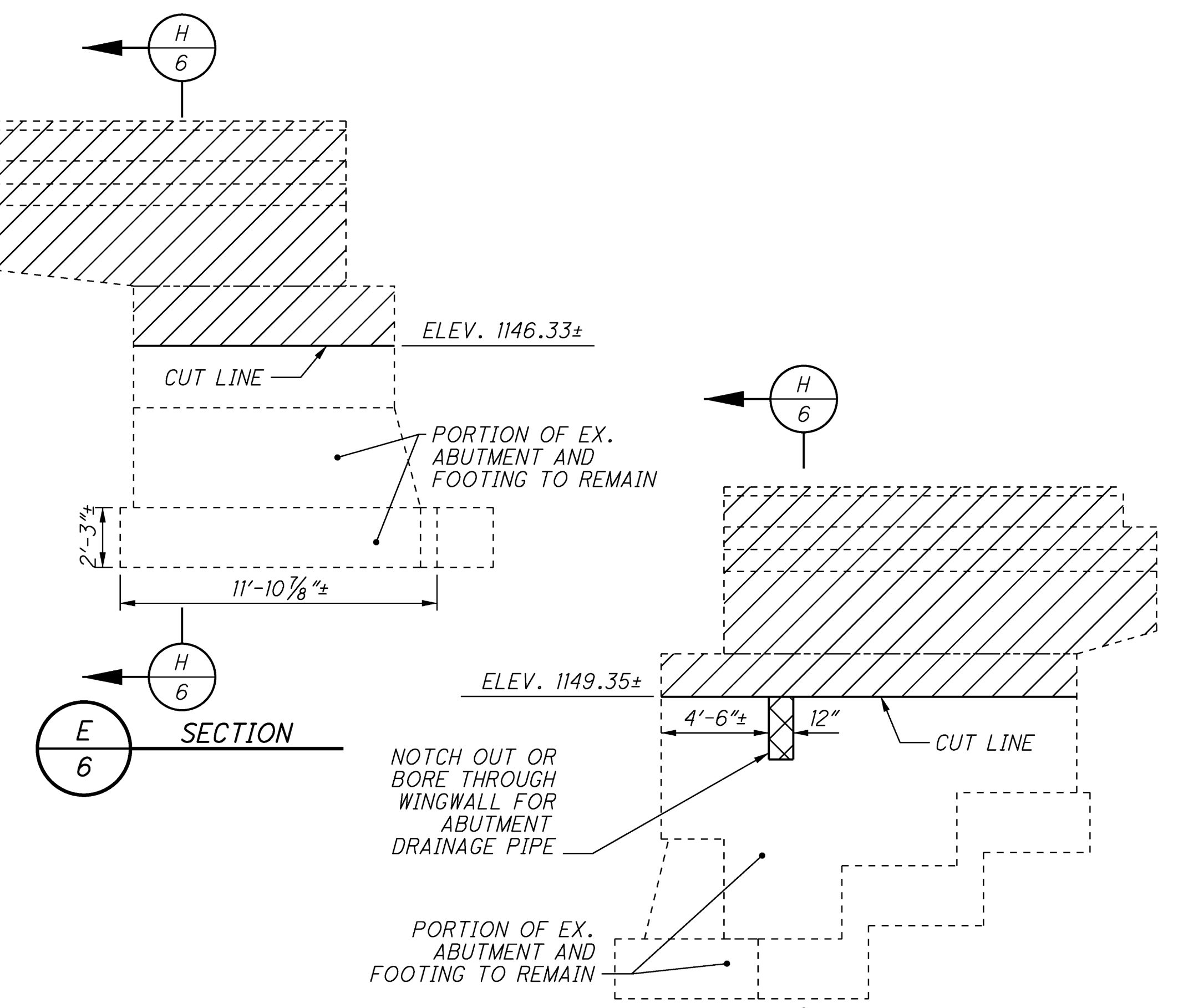
ELEVATION TABLE

	A	B	C	D	E	F
REAR ABUTMENT	1149.48±	1149.76±	1149.13±	1148.51±	1147.88±	1147.19±

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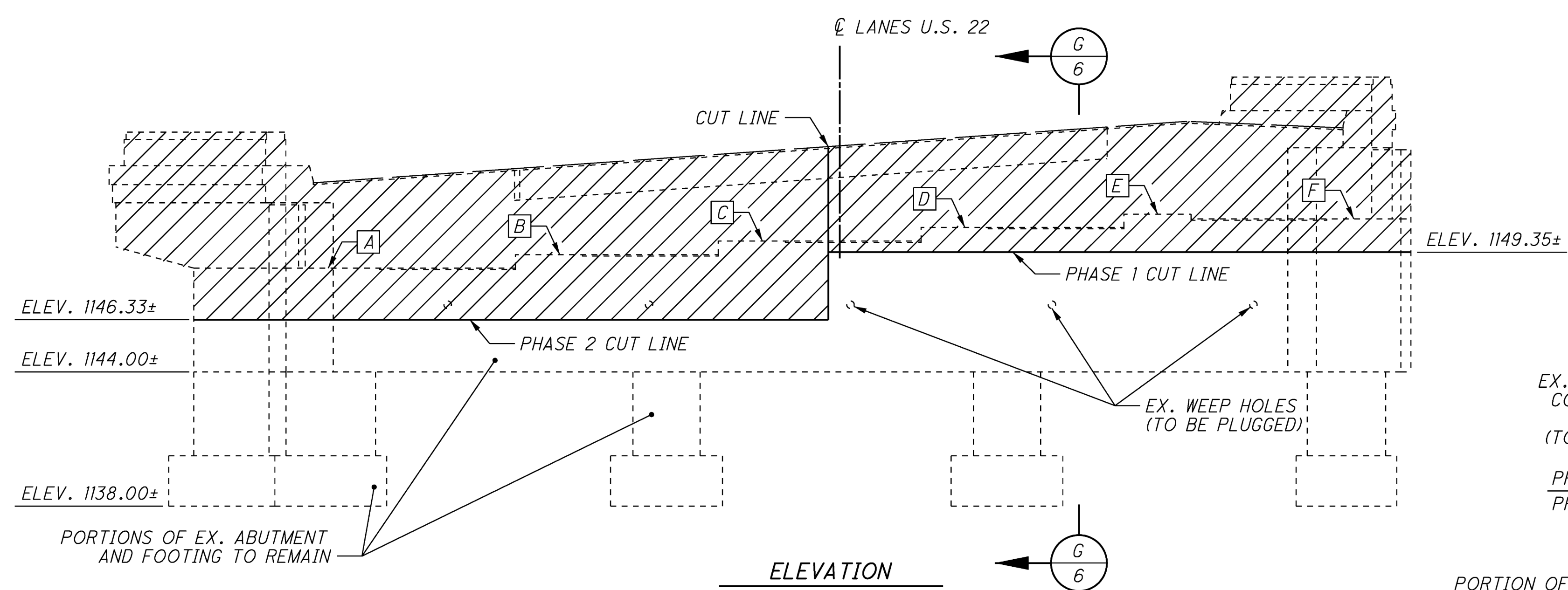


PLAN

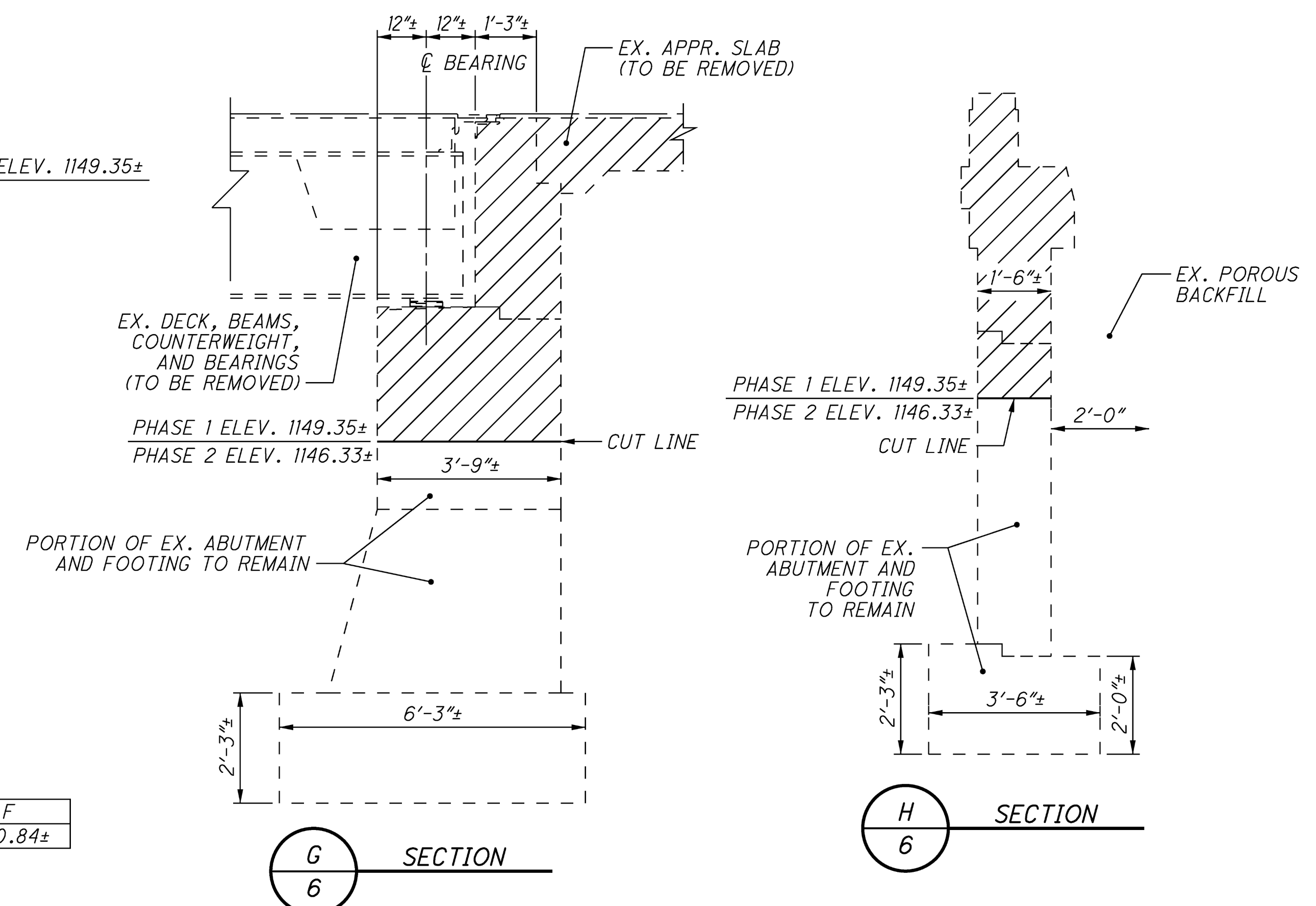


SECTION E-H

SECTION F-H



ELEVATION



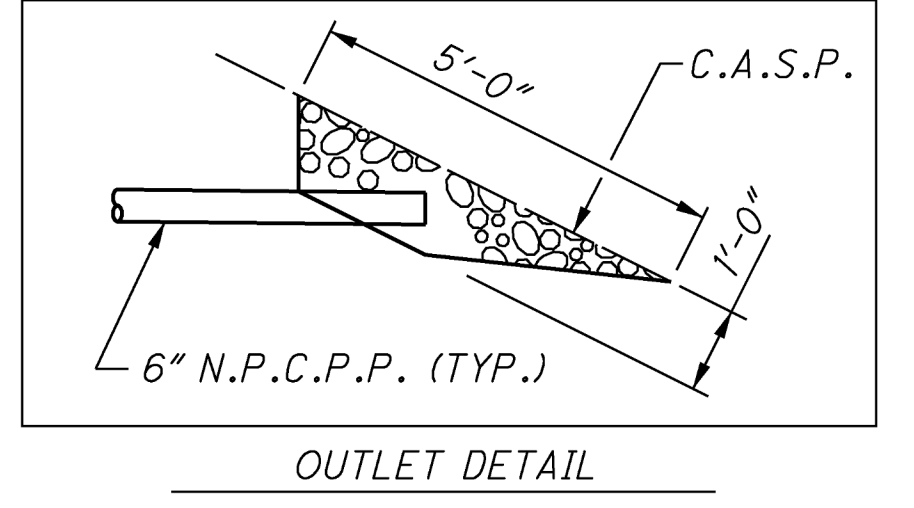
SECTION G-H

SECTION H-H

- PORTIONS OF STRUCTURE TO BE REMOVED
- PORTION OF WINGWALL REMOVED TO PLACE ABUTMENT DRAINAGE PIPE

ELEVATION TABLE

	A	B	C	D	E	F
FWD. ABUTMENT	1148.64±	1149.24±	1149.85±	1150.46±	1151.05±	1150.84±



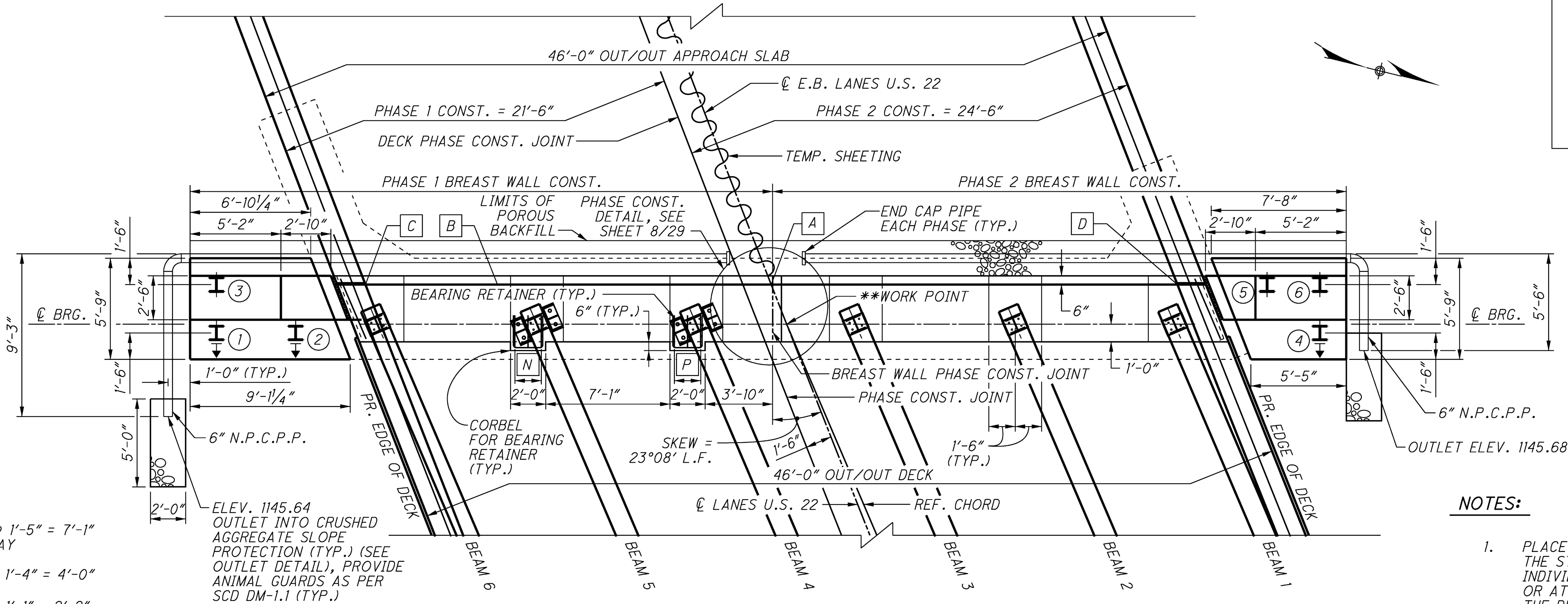
MINIMUM DOWEL DEPTH	
NO. 5 BAR = 1'-0"	
NO. 8 BAR = 1'-6"	

MINIMUM LAP LENGTH	
NO. 5 BAR = 29"	
NO. 8 BAR = 59"	

ELEV. PT. A	1153.02
ELEV. PT. B	1154.14
ELEV. PT. C	1153.81
ELEV. PT. D	1151.36

- [E] 12-A501, 6-A502 SPA. @ 1'-5" = 7'-1" TYP. EXCEPT CENTER BAY
- [F] 8-A501, 4-A502 SPA. @ 1'-4" = 4'-0"
- [G] 6-A501, 3-A502 SPA. @ 1'-1" = 2'-2"
- [H] 1 SERIES OF 10-A503#, SPA. @ 1'-6" = 13'-6", E.F.
- [I] 6-A504# SPA. @ 1'-5" = 7'-6", E.F.
- [J] 1 SERIES OF 15-A507#, SPA. @ 1'-6" = 21'-0", E.F.
- [K] 3-A521 & A523 @ 1'-6" = 3'-0"
- [L] 5-A526 @ 1'-6" = 6'-0" (LAP A523)

- [M] 3-A526 @ 1'-6" = 3'-0" (LAP A540)
- [N] 3-A580 @ 9" = 1'-6"
- [P] 3-A579 @ 9" = 1'-6"

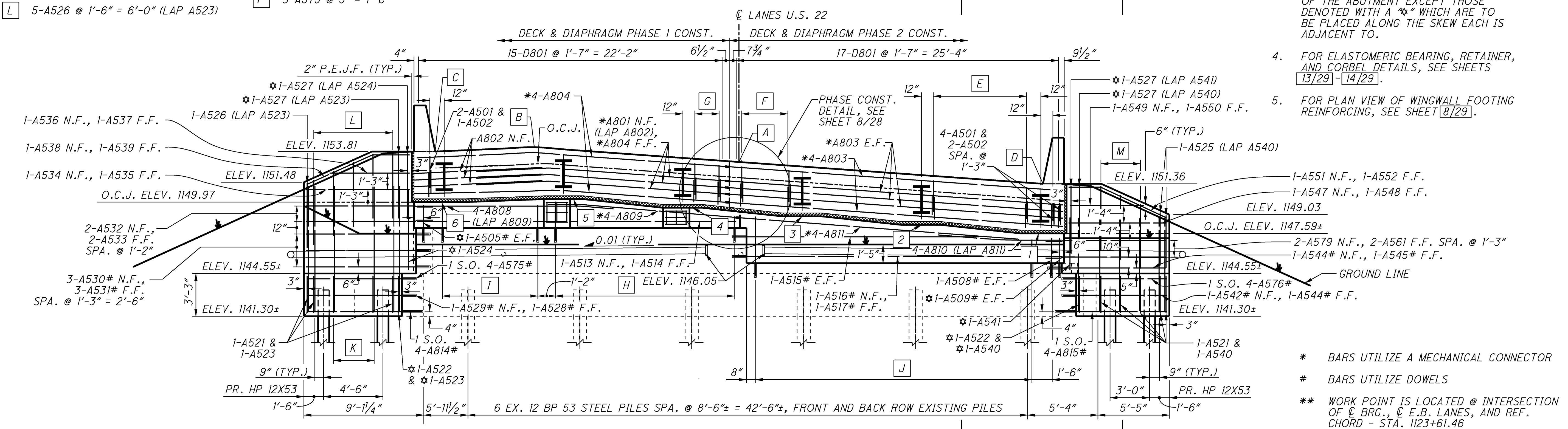


BEAM SEAT ELEVATIONS	
1	ELEV. = 1147.59
2	ELEV. = 1148.24
3	ELEV. = 1148.89
4	ELEV. = 1149.54
5	ELEV. = 1150.20
6	ELEV. = 1149.97

NOTES:

- PLACE THE DIAPHRAGM CONCRETE ENCASEING THE STRUCTURAL MEMBER ENDS OF AN INDIVIDUAL PHASE WITH THE DECK CONCRETE OR AT LEAST 48 HOURS BEFORE PLACEMENT OF THE DECK CONCRETE. IF PLACED SEPARATELY, LOCATE THE HORIZONTAL CONSTRUCTION JOINT BETWEEN THE DIAPHRAGM AND DECK CONCRETE AT THE APPROACH SLAB SEAT.
- ALL A501, A502, AND D801 BARS ARE MEASURED ALONG C/BRG AND PLACED PARALLEL TO BEAMS.
- ALL VERTICAL BREASTWALL BARS, WING WALL BARS, AND FOOTING BARS ARE TO BE PLACED NORMAL TO THE FRONT FACE OF THE ABUTMENT EXCEPT THOSE DENOTED WITH A "*" WHICH ARE TO BE PLACED ALONG THE SKEW EACH IS ADJACENT TO.
- FOR ELASTOMERIC BEARING, RETAINER, AND CORBEL DETAILS, SEE SHEETS 13/29-14/29.
- FOR PLAN VIEW OF WINGWALL FOOTING REINFORCING, SEE SHEET 8/29.

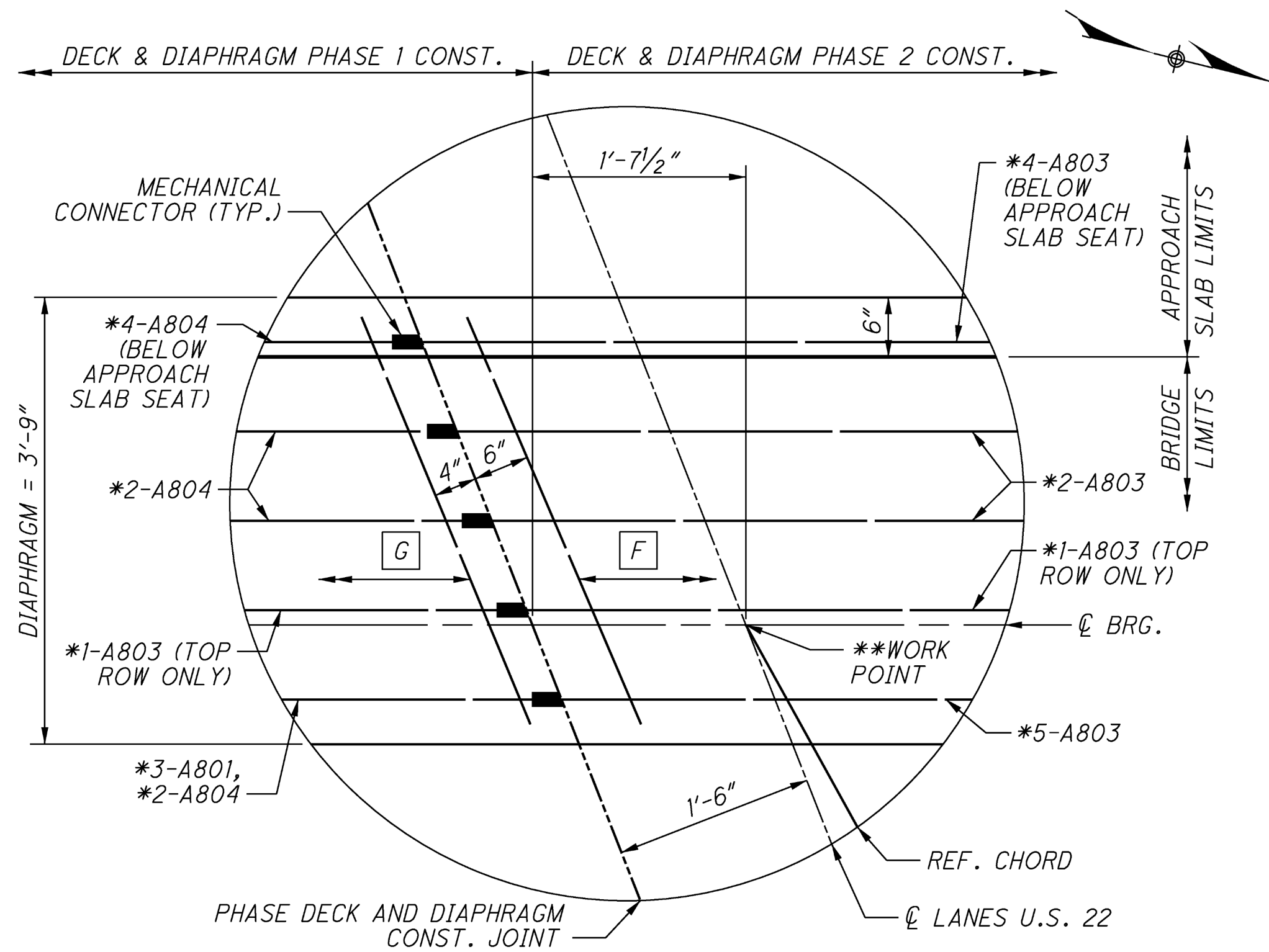
PLAN



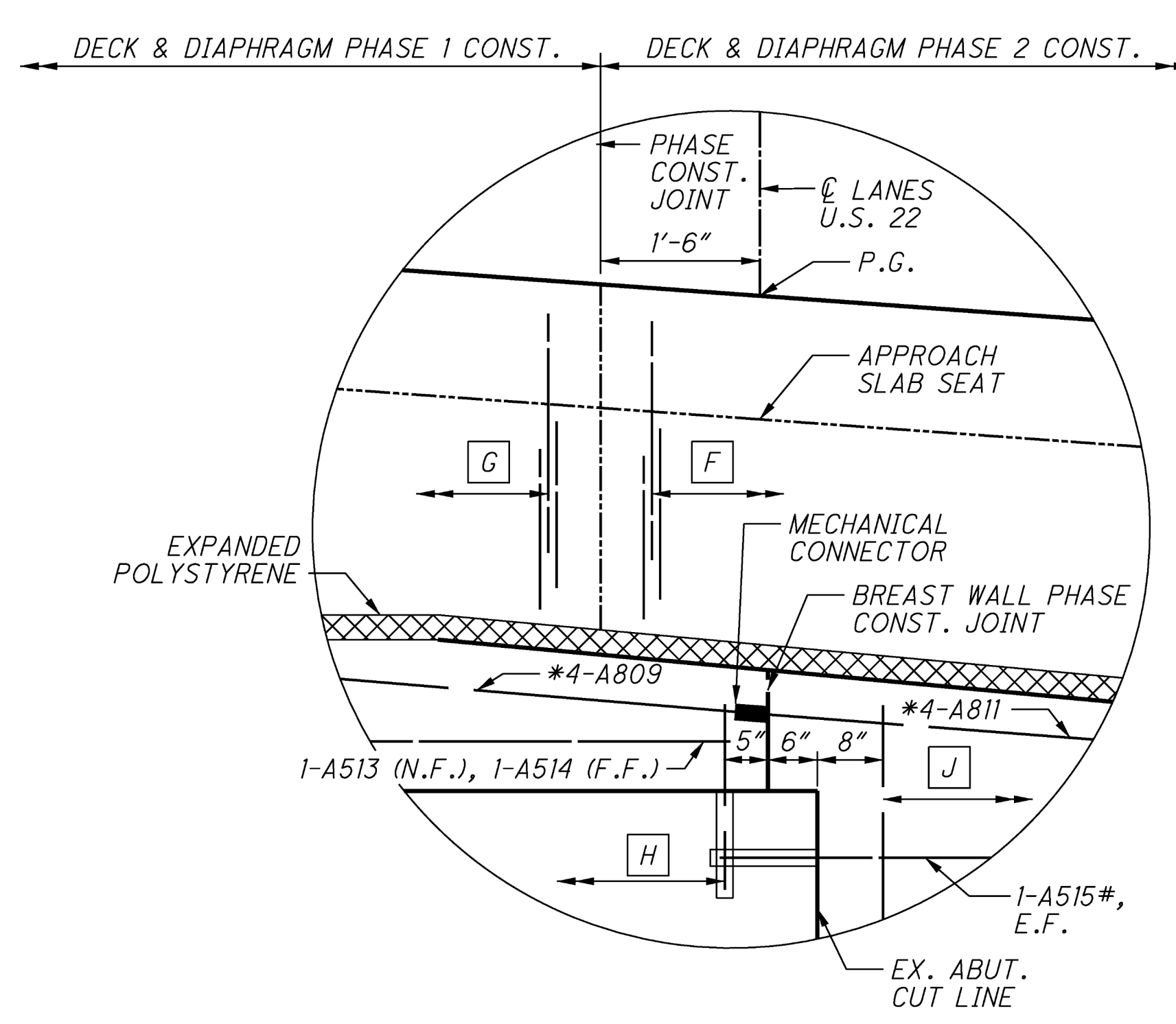
ELEVATION

- * BARS UTILIZE A MECHANICAL CONNECTOR
- # BARS UTILIZE DOWELS
- ** WORK POINT IS LOCATED @ INTERSECTION OF C/BRG., C/E.B. LANES, AND REF. CHORD - STA. 1123+61.46
- (X) PILE NUMBER
- ↓ PILE BATTER (4:1)

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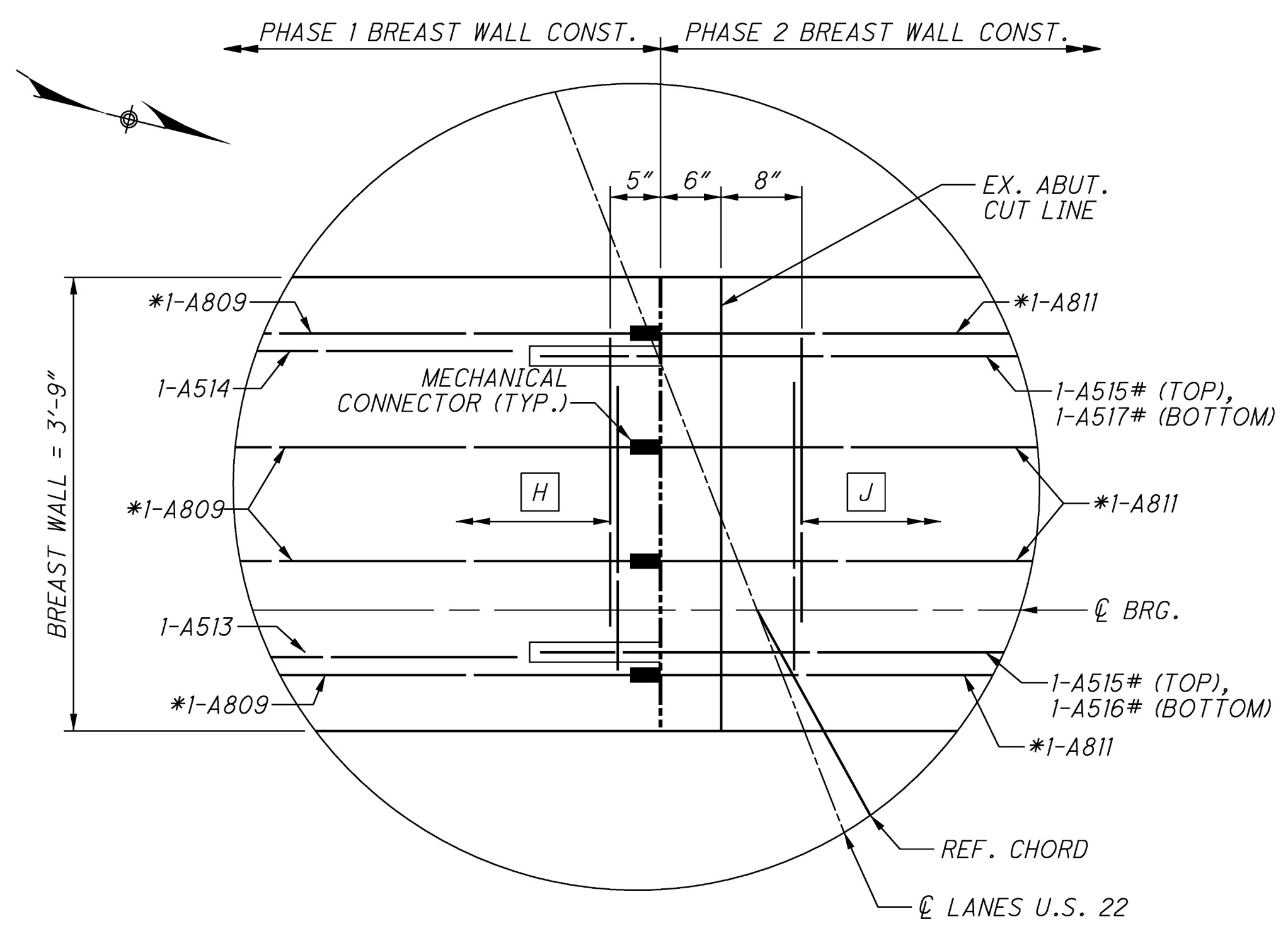


PHASE CONSTRUCTION DETAILS - PLAN VIEW
DIAPHRAGM SECTION

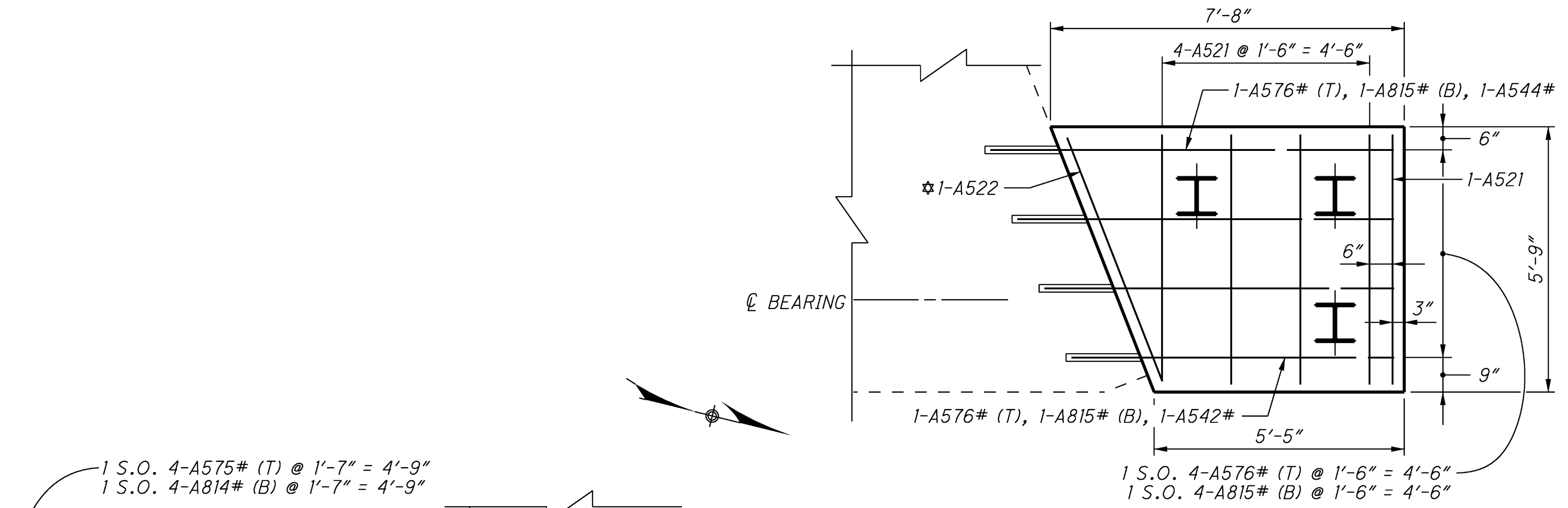


PHASE CONSTRUCTION DETAILS - ELEVATION VIEW

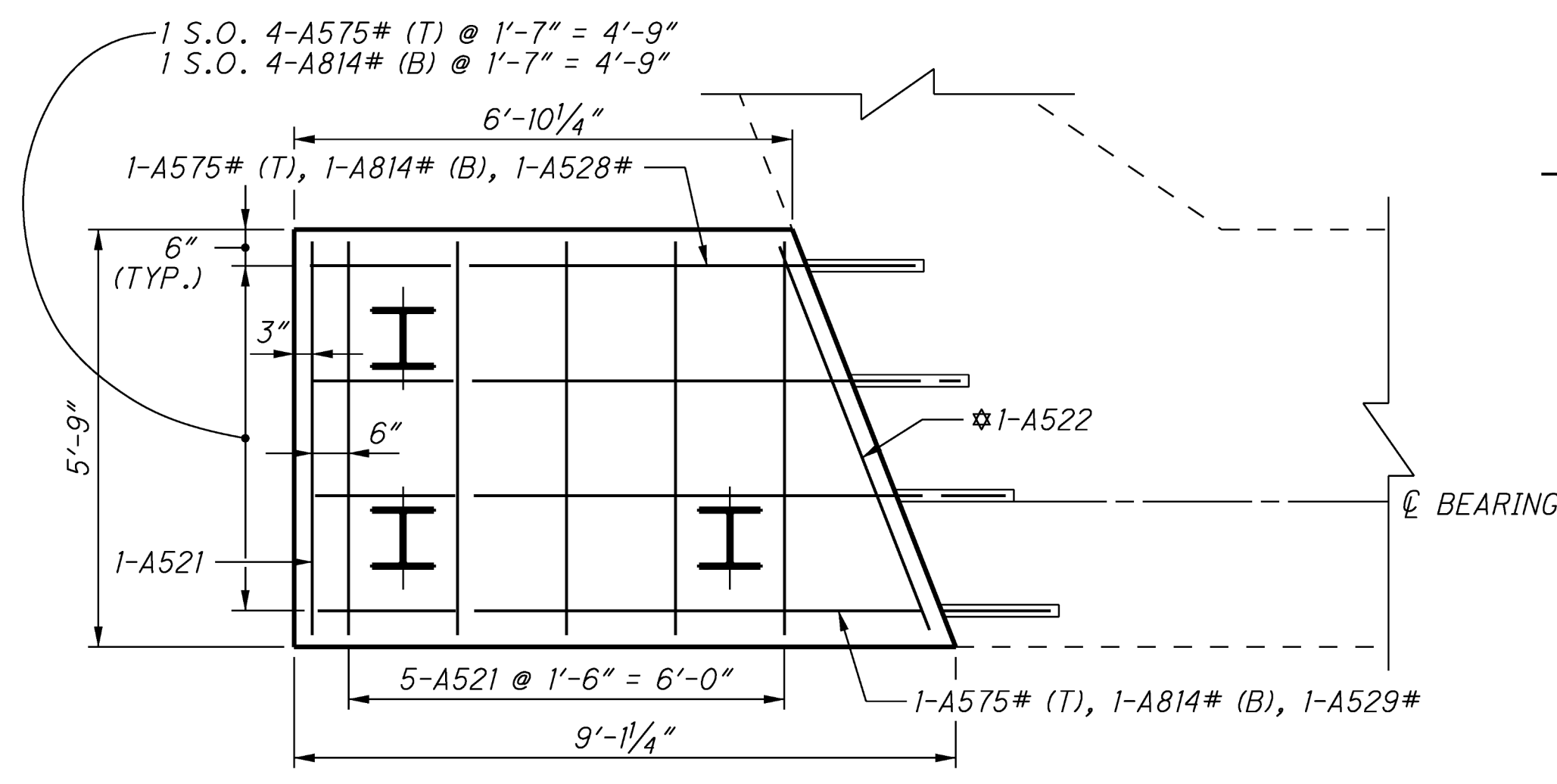
- NOTES:**
- DIAPHRAGM CONSTRUCTION JOINT IS PARALLEL TO BEAMS. BREAST WALL CONSTRUCTION JOINT IS NORMAL TO BREAST WALL.
 - FOR REINFORCING REFERENCES, SEE SHEET [7/29].
 - APPLY TYPE 2 WATERPROOFING 3'-0" WIDE CENTERED ABOUT THE VERTICAL CONST. JOINT BETWEEN THE RIGHT WING AND PR. BREASTWALL, THE VERTICAL BREASTWALL CONST. JOINT BETWEEN PHASES, AND THE HORIZONTAL CONST. JOINT BETWEEN THE EX. AND PR. BREASTWALL ON PHASE 1 ONLY.
- * BARS UTILIZE A MECHANICAL CONNECTOR
BARS UTILIZE DOWELS
** WORK POINT IS LOCATED @ INTERSECTION OF CL BRG., CL E.B. LANES, AND REF. CHORD. STA. 1123+61.46



PHASE CONSTRUCTION DETAILS - PLAN VIEW
BREAST WALL SECTION



LEFT WINGWALL FOOTING - PLAN VIEW



RIGHT WINGWALL FOOTING - PLAN VIEW

DESIGN AGENCY O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING	DATE	REVIEWED	STRUCTURE FILE NUMBER 340200
DRAWN CCN	CHECKED RPT	DESIGNED CCN	REVISED
REAR ABUTMENT DETAILS BRIDGE NO. HAS-22-2126 OVER WHEELING & LAKE ERIE RAILROAD			
HAS-22-21.26 PID No. 88904		8 / 29	
31 52			

MINIMUM DOWEL DEPTH	
NO. 5 BAR = 1'-0"	
NO. 8 BAR = 1'-6"	

MINIMUM LAP LENGTH	
NO. 5 BAR = 29"	
NO. 8 BAR = 59"	

ELEV. PT. A	1154.43
ELEV. PT. B	1155.54
ELEV. PT. C	1155.21
ELEV. PT. D	1152.79

- E 12-A501, 6-A502 SPA. @ 1'-5" = 7'-1" TYP. EXCEPT CENTER BAY
- F 8-A501, 4-A502 SPA. @ 1'-4" = 4'-0"
- G 6-A501, 3-A502 SPA. @ 1'-0" = 2'-0"
- H 1 SERIES OF 10-A510# SPA. @ 1'-6" = 13'-6", E.F.
- J 7-A504# SPA. @ 1'-6" = 9'-0", E.F.
- K 1 SERIES OF 16-A511# SPA. @ 1'-6" = 22'-6", E.F.
- L 5-A553 & A555 @ 1'-6" = 6'-0"
- M 5-A526 @ 1'-6" = 6'-0" (LAP A555)
- N 5-A553 & A567 @ 1'-6" = 7'-6"

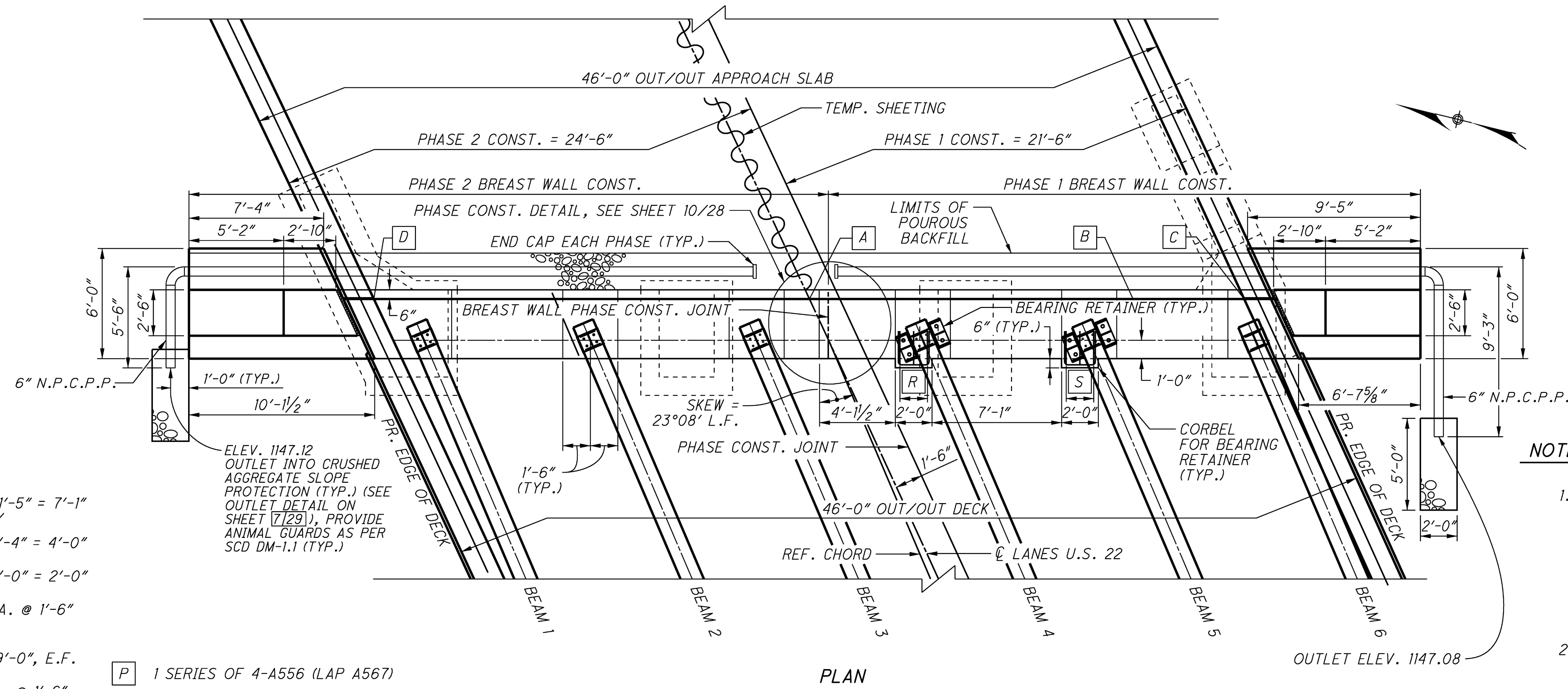
- P 1 SERIES OF 4-A556 (LAP A567)
- Q 2-A526 @ 1'-6" = 3'-0" (LAP A567)
- R 3-A579 @ 9" = 1'-6"
- S 3-A580 @ 9" = 1'-6"

BEAM SEAT ELEVATIONS

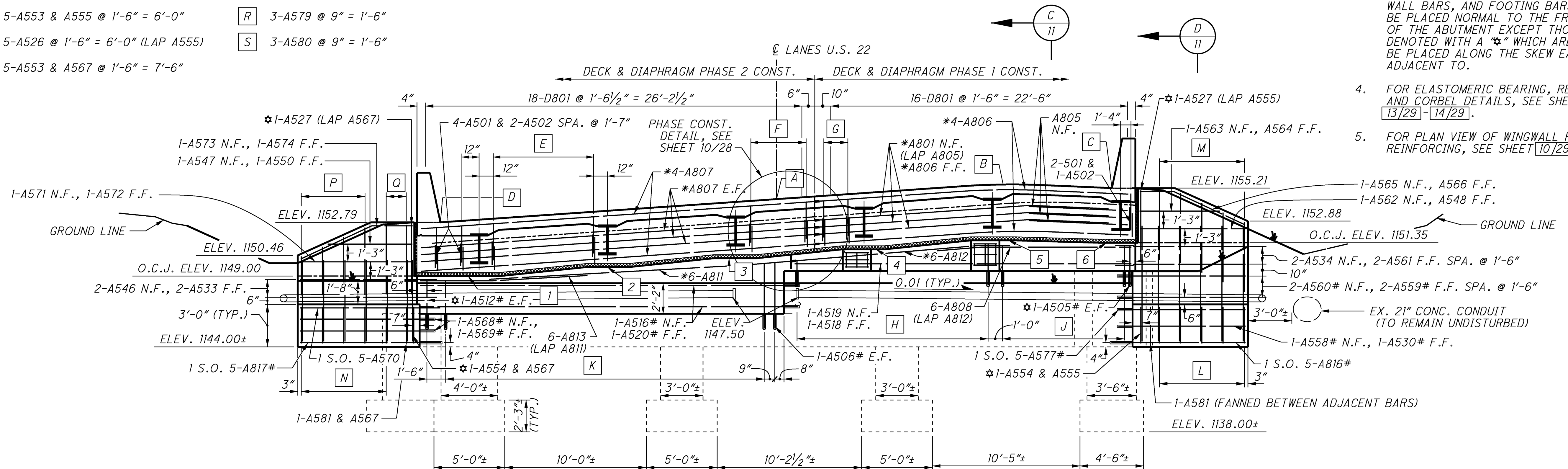
- 1 ELEV. = 1149.00
- 2 ELEV. = 1149.63
- 3 ELEV. = 1150.26
- 4 ELEV. = 1150.89
- 5 ELEV. = 1151.52
- 6 ELEV. = 1151.35

NOTES:

- PLACE THE DIAPHRAGM CONCRETE ENCASING THE STRUCTURAL MEMBER ENDS OF AN INDIVIDUAL PHASE WITH THE DECK CONCRETE OR AT LEAST 48 HOURS BEFORE PLACEMENT OF THE DECK CONCRETE. IF PLACED SEPARATELY, LOCATE THE HORIZONTAL CONSTRUCTION JOINT BETWEEN THE DIAPHRAGM AND DECK CONCRETE AT THE APPROACH SLAB SEAT.
- ALL A501, A502, AND D801 BARS ARE MEASURED ALONG C BRG AND PLACED PARALLEL TO BEAMS.
- ALL VERTICAL BREAST WALL BARS, WING WALL BARS, AND FOOTING BARS ARE TO BE PLACED NORMAL TO THE FRONT FACE OF THE ABUTMENT EXCEPT THOSE DENOTED WITH A * WHICH ARE TO BE PLACED ALONG THE SKEW EACH IS ADJACENT TO.
- FOR ELASTOMERIC BEARING, RETAINER, AND CORBEL DETAILS, SEE SHEETS 13/29-14/29.
- FOR PLAN VIEW OF WINGWALL FOOTING REINFORCING, SEE SHEET 10/29.



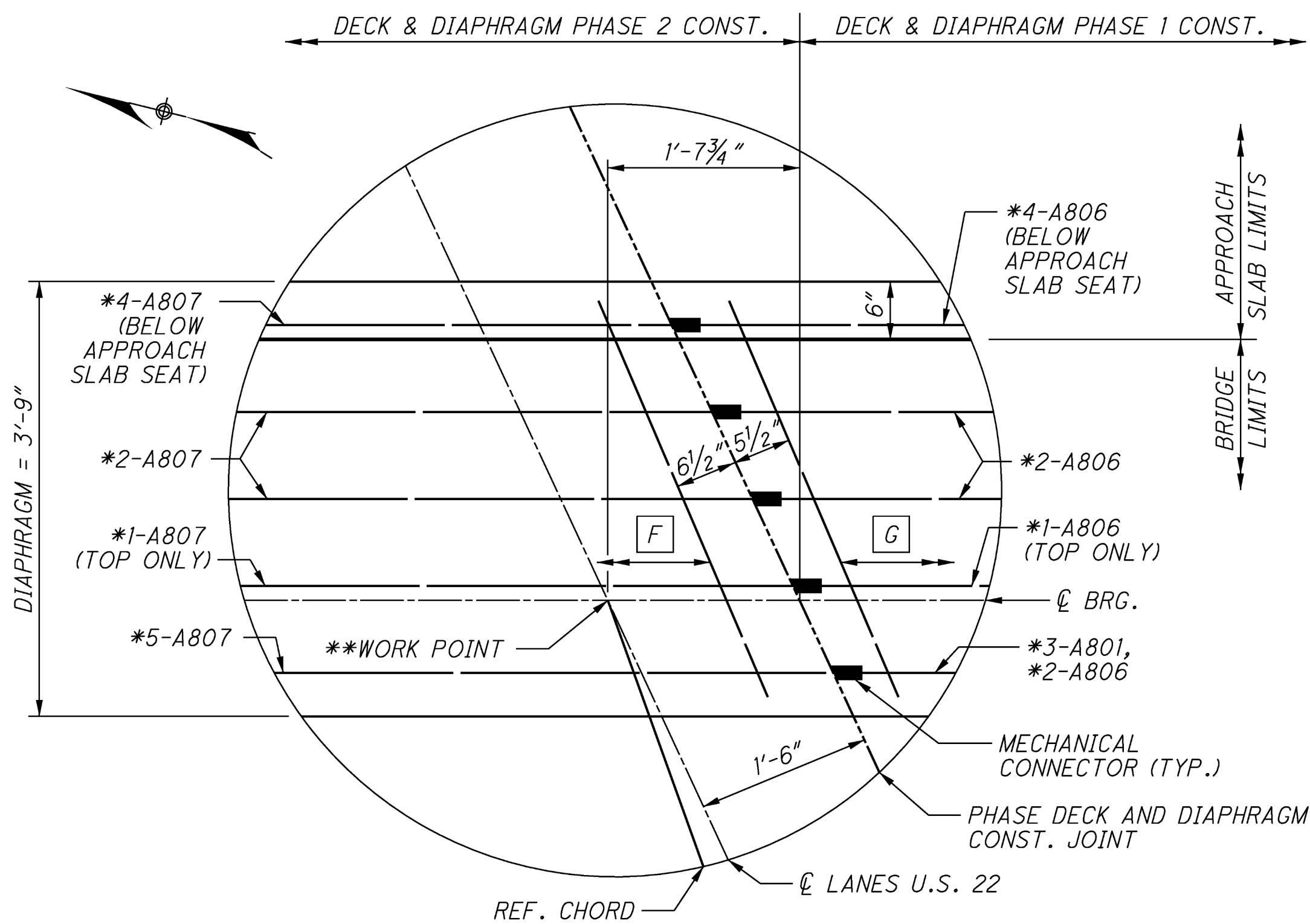
PLAN



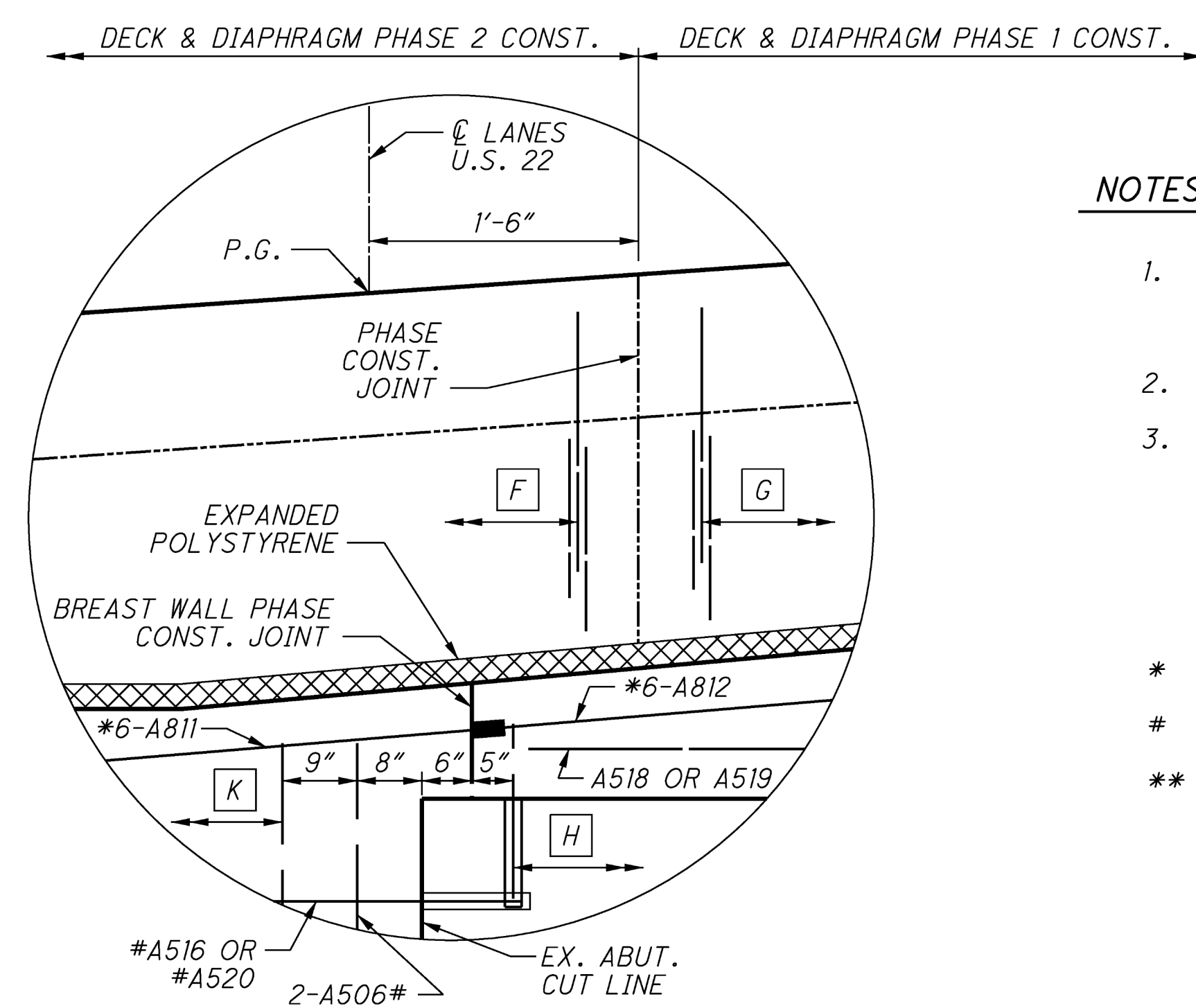
ELEVATION

- * BARS UTILIZE A MECHANICAL CONNECTOR
- # BARS UTILIZE DOWELS
- ** WORK POINT IS LOCATED @ INTERSECTION OF C BRG., E.B. LANES, AND REF. CHORD. STA. 1125+03.13

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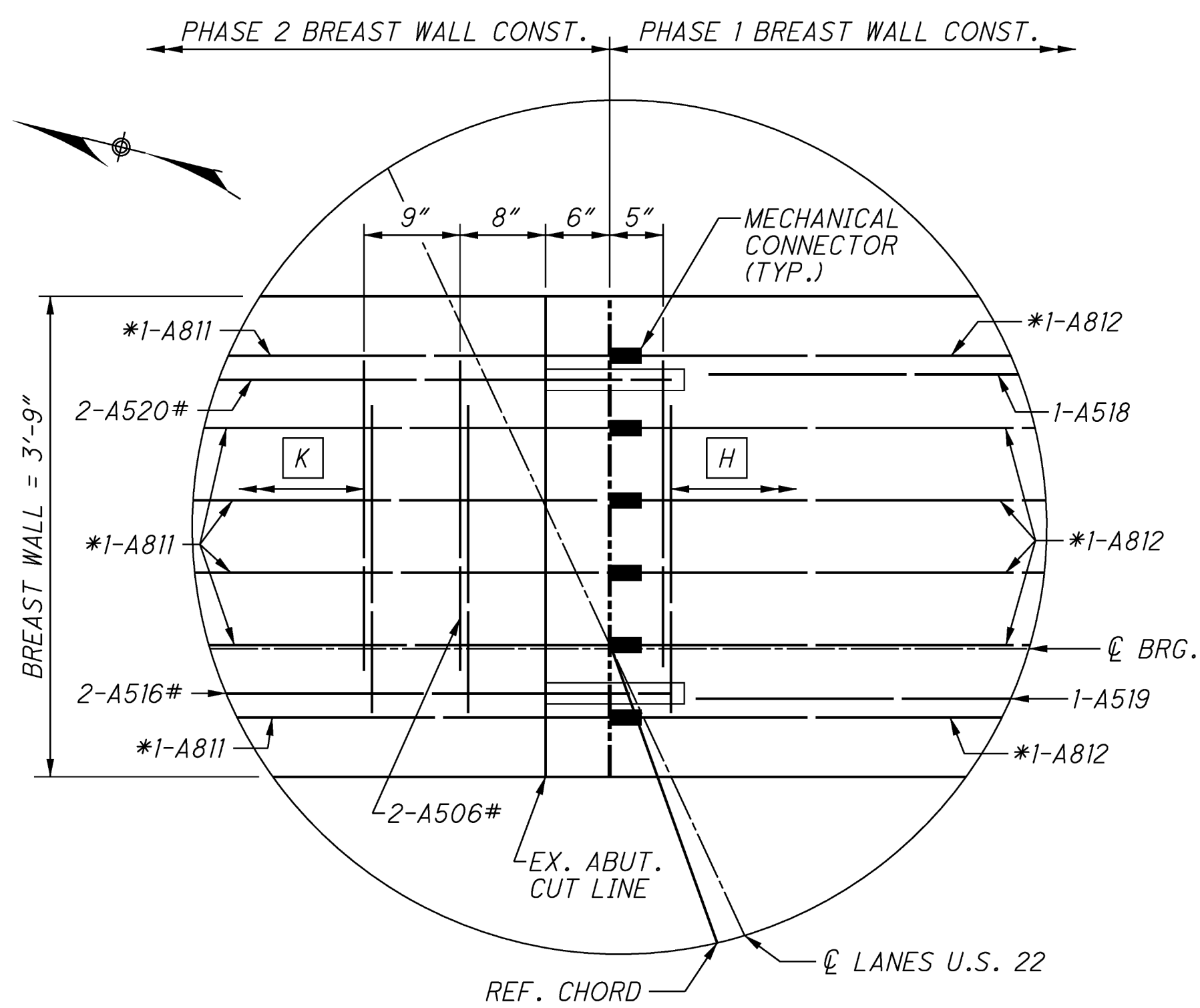
PHASE CONSTRUCTION DETAILS - PLAN VIEW
DIAPHRAGM SECTION



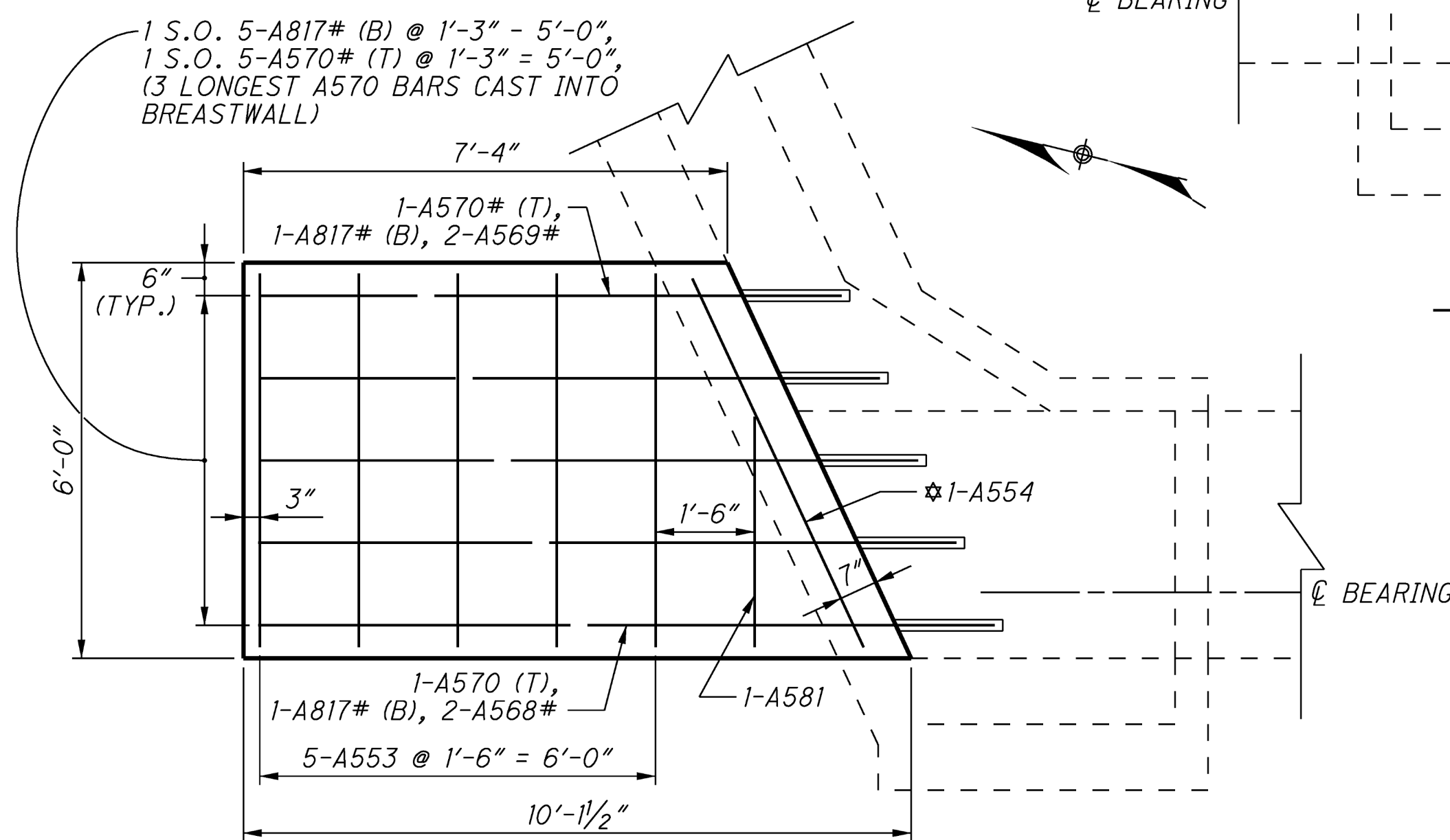
PHASE CONSTRUCTION DETAILS - ELEVATION VIEW

NOTES:

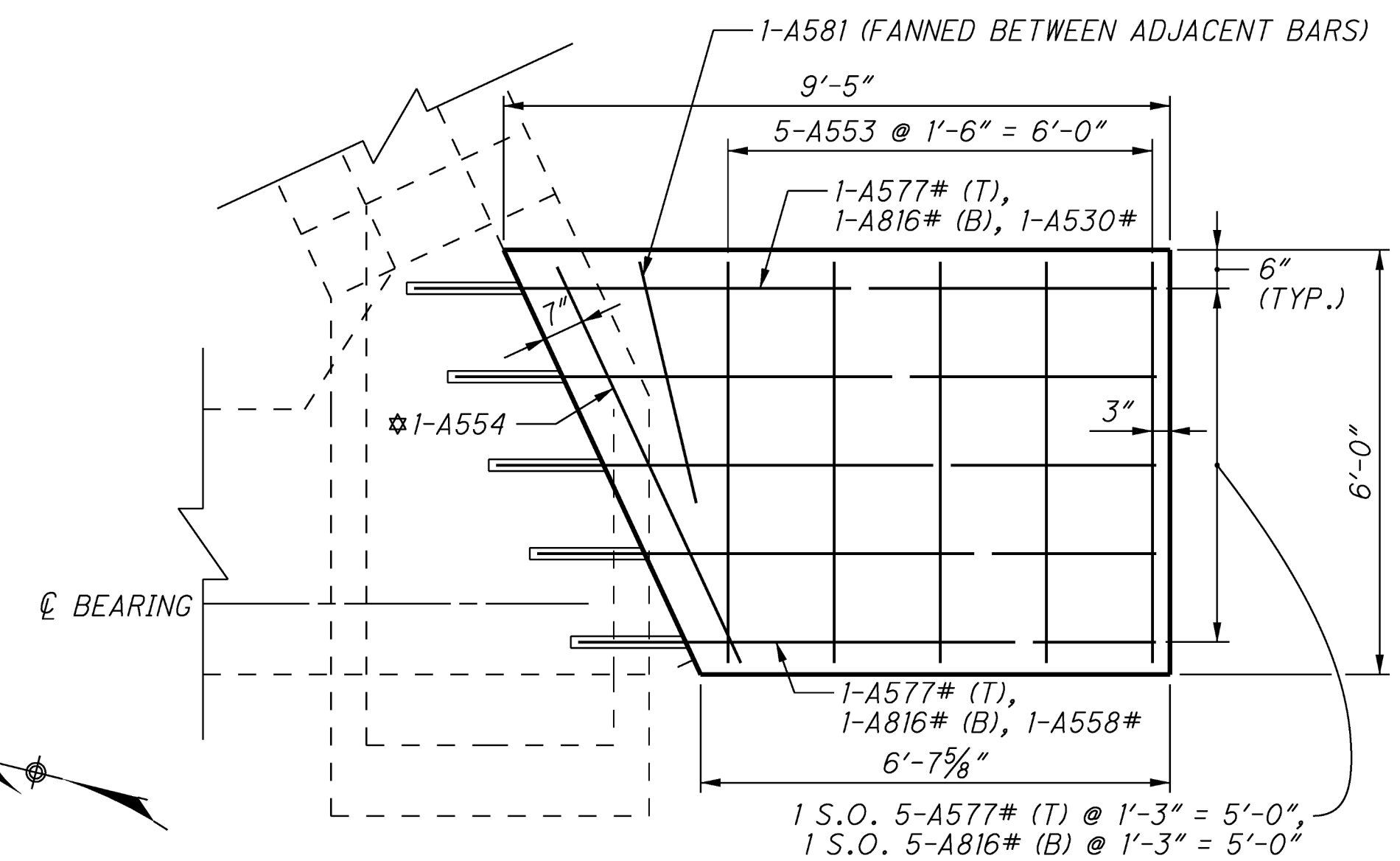
- DIAPHRAGM CONSTRUCTION JOINT IS PARALLEL TO BEAMS. BREAST WALL CONSTRUCTION JOINT IS NORMAL TO BREAST WALL.
 - FOR REINFORCING REFERENCES, SEE SHEET 9/29.
 - APPLY TYPE 2 WATERPROOFING 3'-0" WIDE CENTERED ABOUT THE VERTICAL CONST. JOINT BETWEEN THE RIGHT WING AND PR. BREASTWALL, THE VERTICAL BREASTWALL CONST. JOINT BETWEEN PHASES, AND THE HORIZONTAL CONST. JOINT BETWEEN THE EX. AND PR. BREASTWALL ON PHASE 1 ONLY.
- * BARS UTILIZE A MECHANICAL CONNECTOR
BARS UTILIZE DOWELS
** WORK POINT IS LOCATED @ INTERSECTION OF CL BRG., CL E.B. LANES, AND REF. CHORD. STA. 1125+03.13



PHASE CONSTRUCTION DETAILS - PLAN VIEW
BREAST WALL SECTION



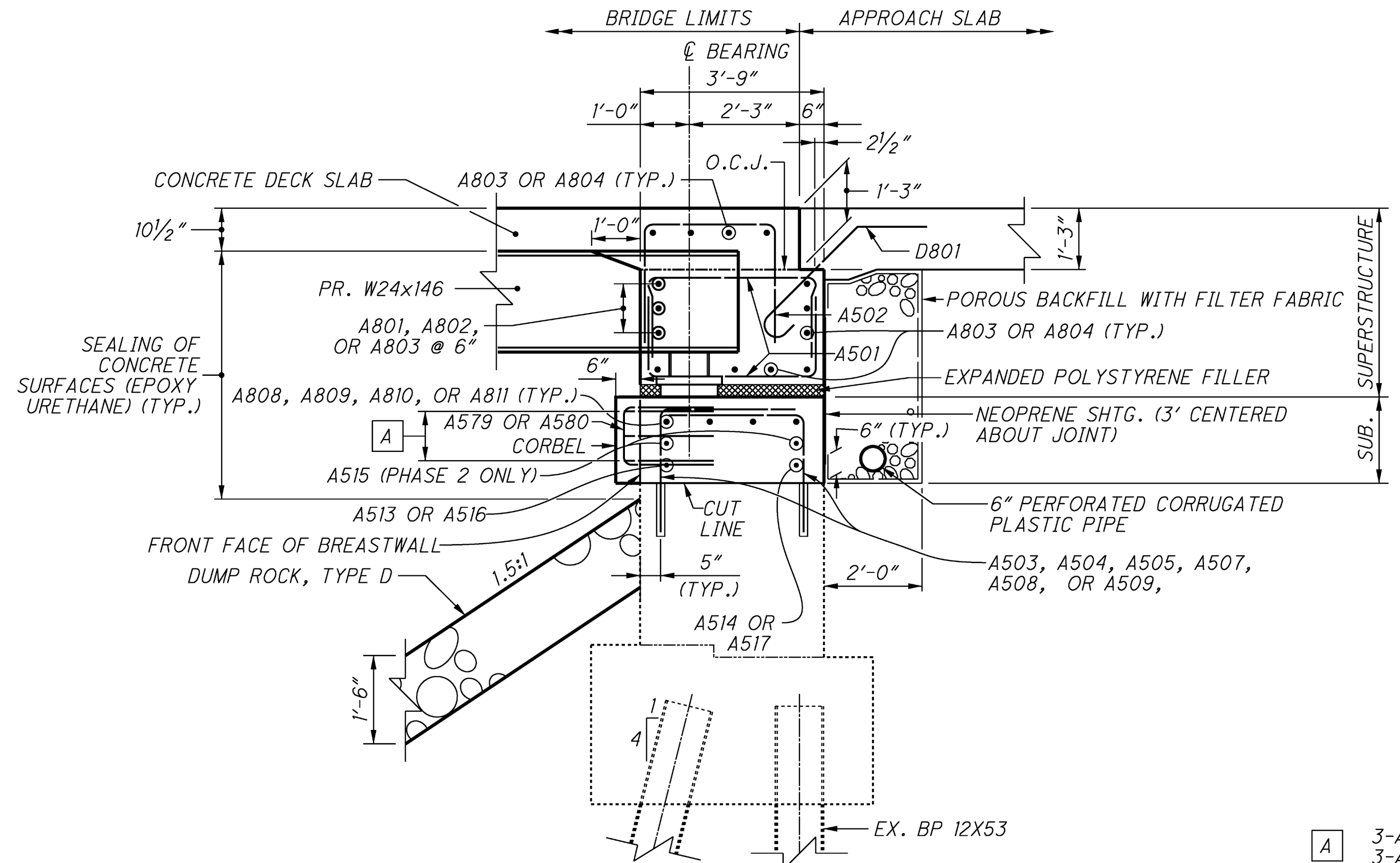
LEFT WINGWALL FOOTING - PLAN VIEW



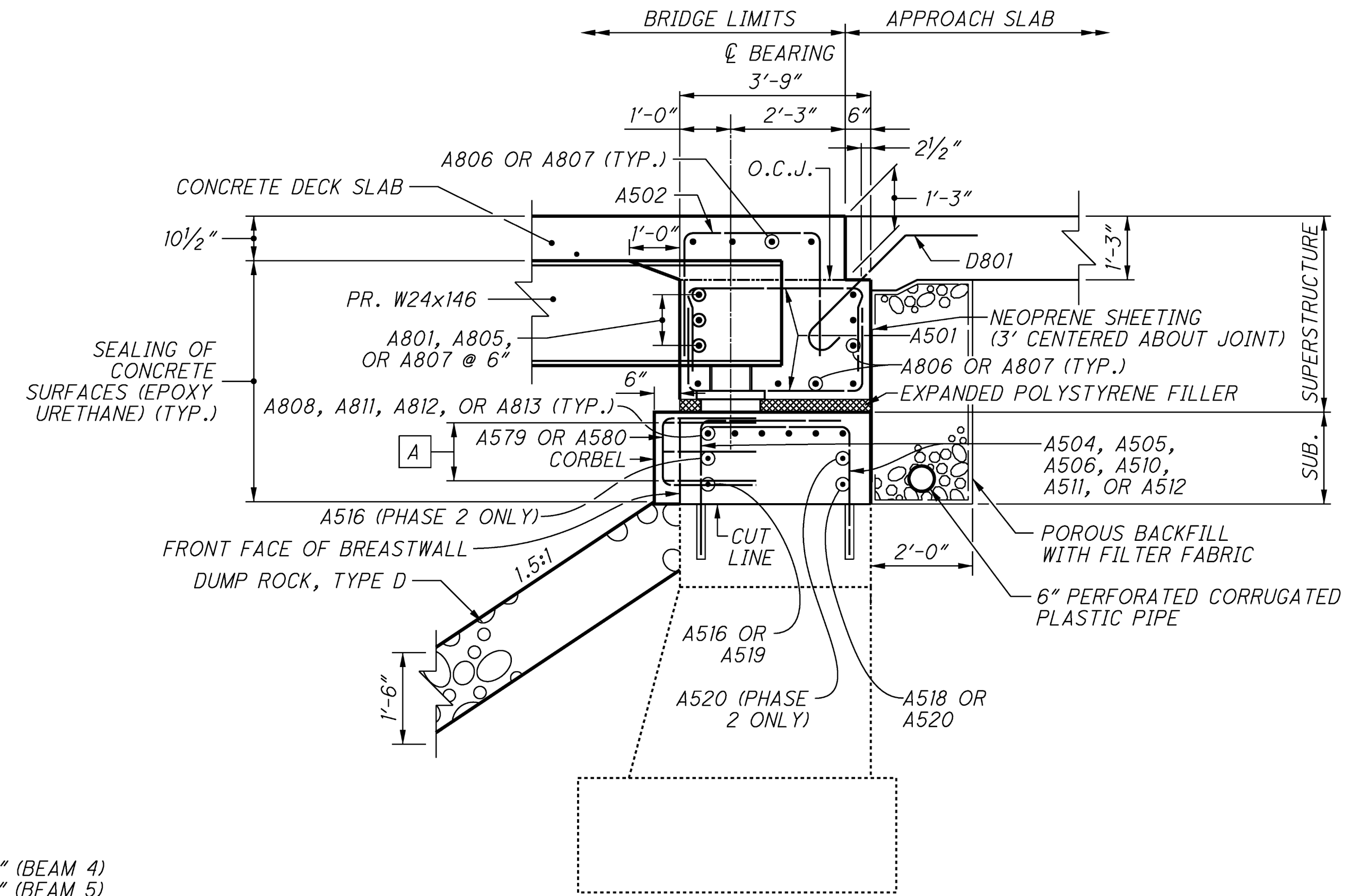
RIGHT WINGWALL FOOTING - PLAN VIEW

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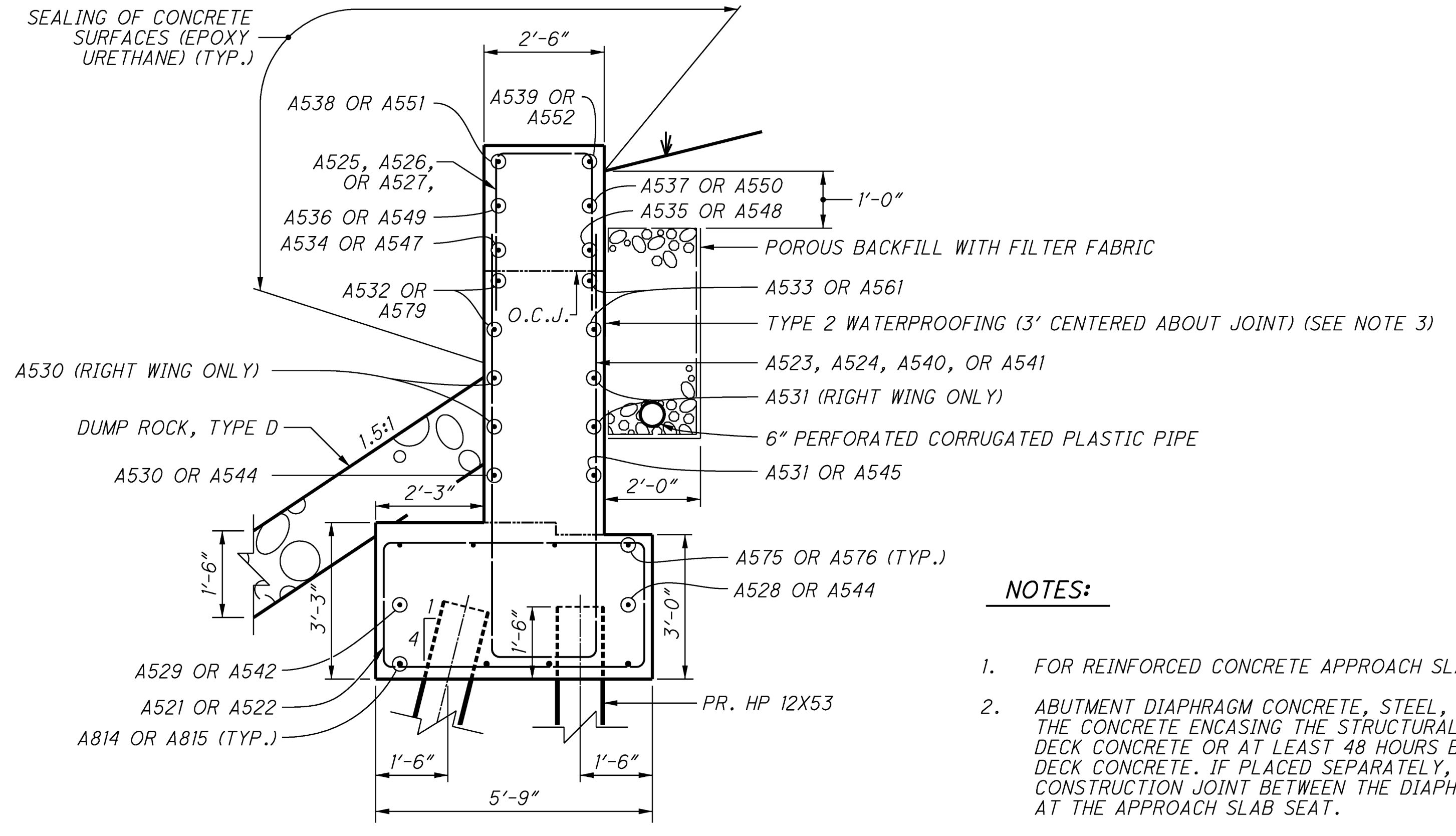
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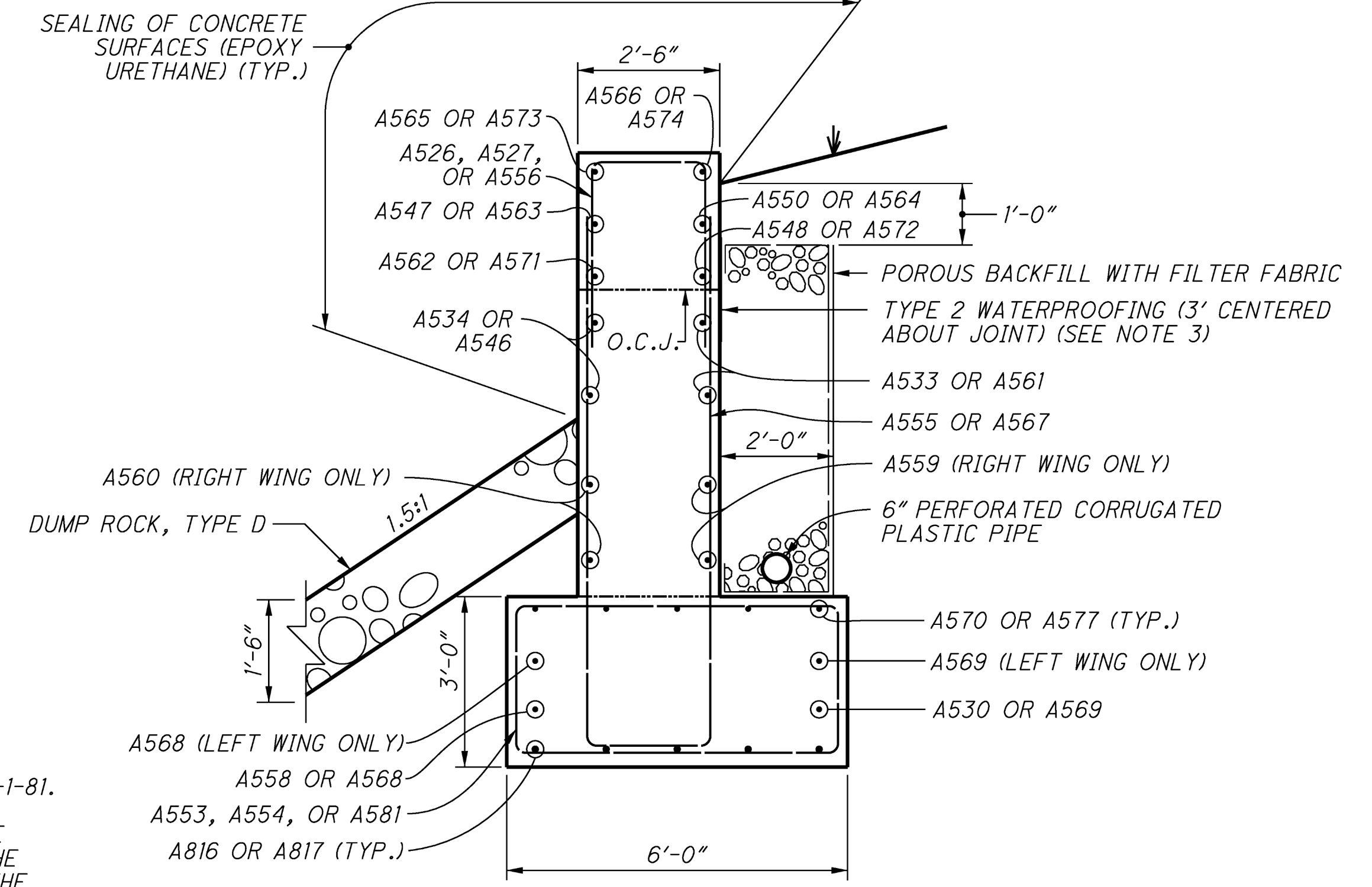
A
7 PR. REAR ABUTMENT SECTION



C
9 PR. FORWARD ABUTMENT SECTION



B
7 PR. REAR WING WALL SECTION

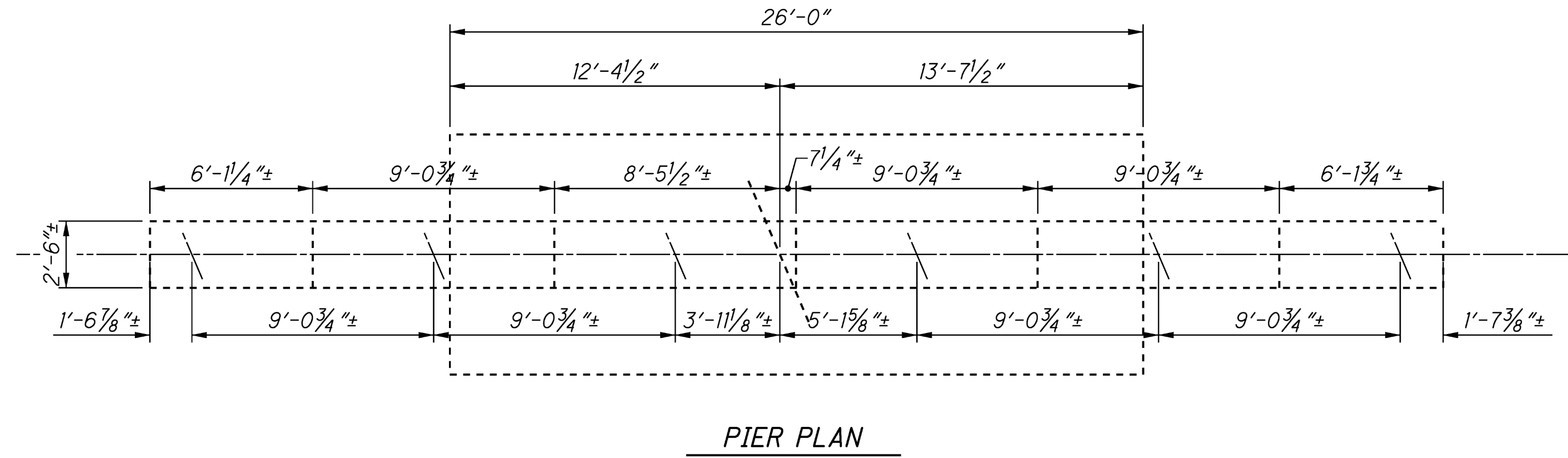


D
9 PR. FORWARD WING WALL SECTION

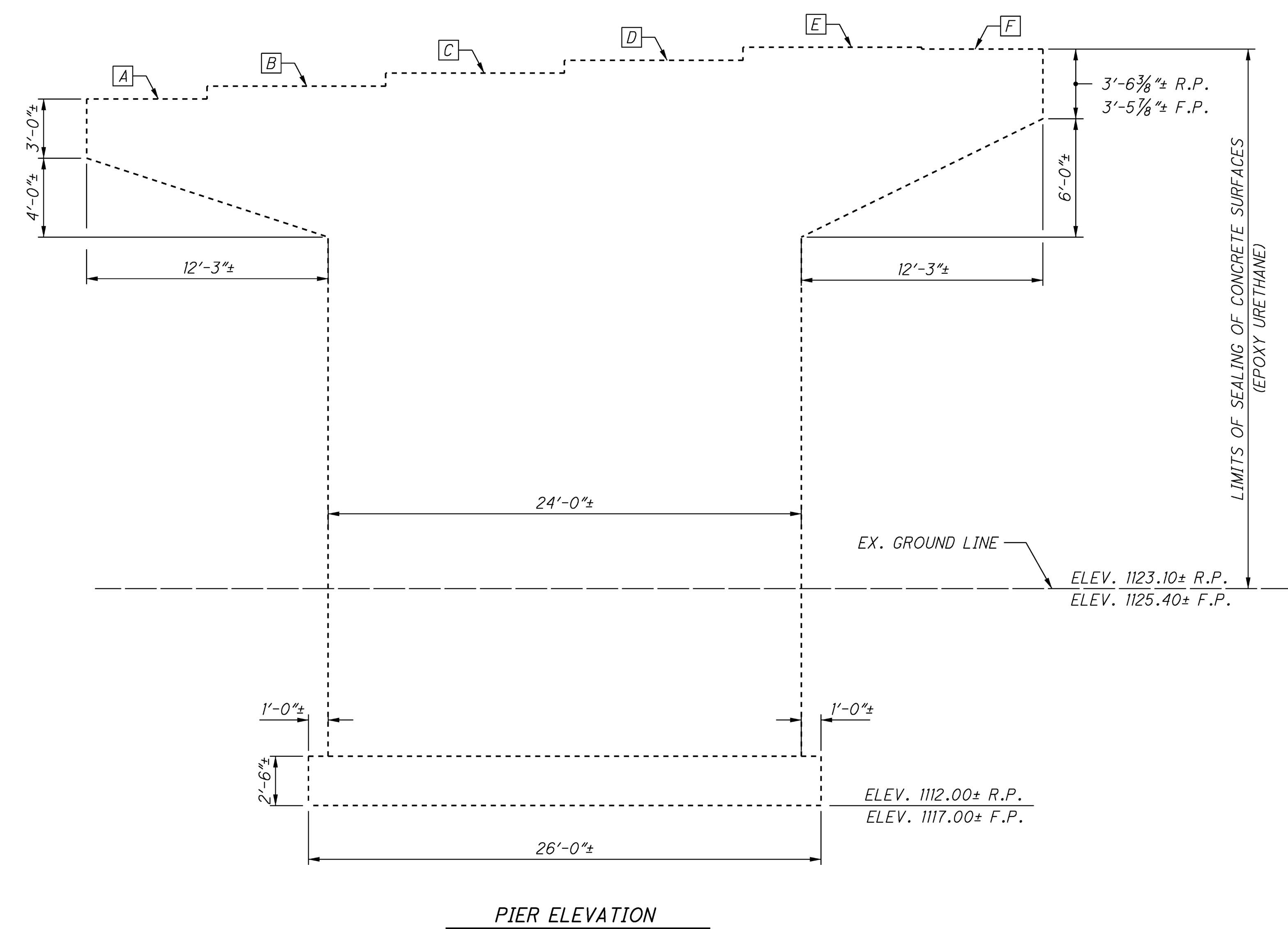
- NOTES:**
- FOR REINFORCED CONCRETE APPROACH SLAB DETAILS SEE SCD AS-1-81.
 - ABUTMENT DIAPHRAGM CONCRETE, STEEL, SUPERSTRUCTURE: PLACE THE CONCRETE ENCASING THE STRUCTURAL STEELMEMBERS WITH THE DECK CONCRETE OR AT LEAST 48 HOURS BEFORE PLACEMENT OF THE DECK CONCRETE. IF PLACED SEPARATELY, LOCATE THE HORIZONTAL CONSTRUCTION JOINT BETWEEN THE DIAPHRAGM AND DECK CONCRETE AT THE APPROACH SLAB SEAT.
 - IF THE OPTIONAL CONSTRUCTION JOINT IS USED ON THE WING WALLS, APPLY TYPE 2 WATERPROOFING 3'-0" WIDE CENTERED ABOUT THE HORIZONTAL CONSTRUCTION JOINT.
 - FOR ELASTOMERIC BEARING DETAILS, SEE SHEETS [13/29] - [14/29].
 - FOR REINFORCING STEEL LIST, SEE SHEET [26/29] - [29/29].

A 3-A578 @ 6" = 1'-0" (BEAM 4)
3-A578 @ 9" = 1'-6" (BEAM 5)

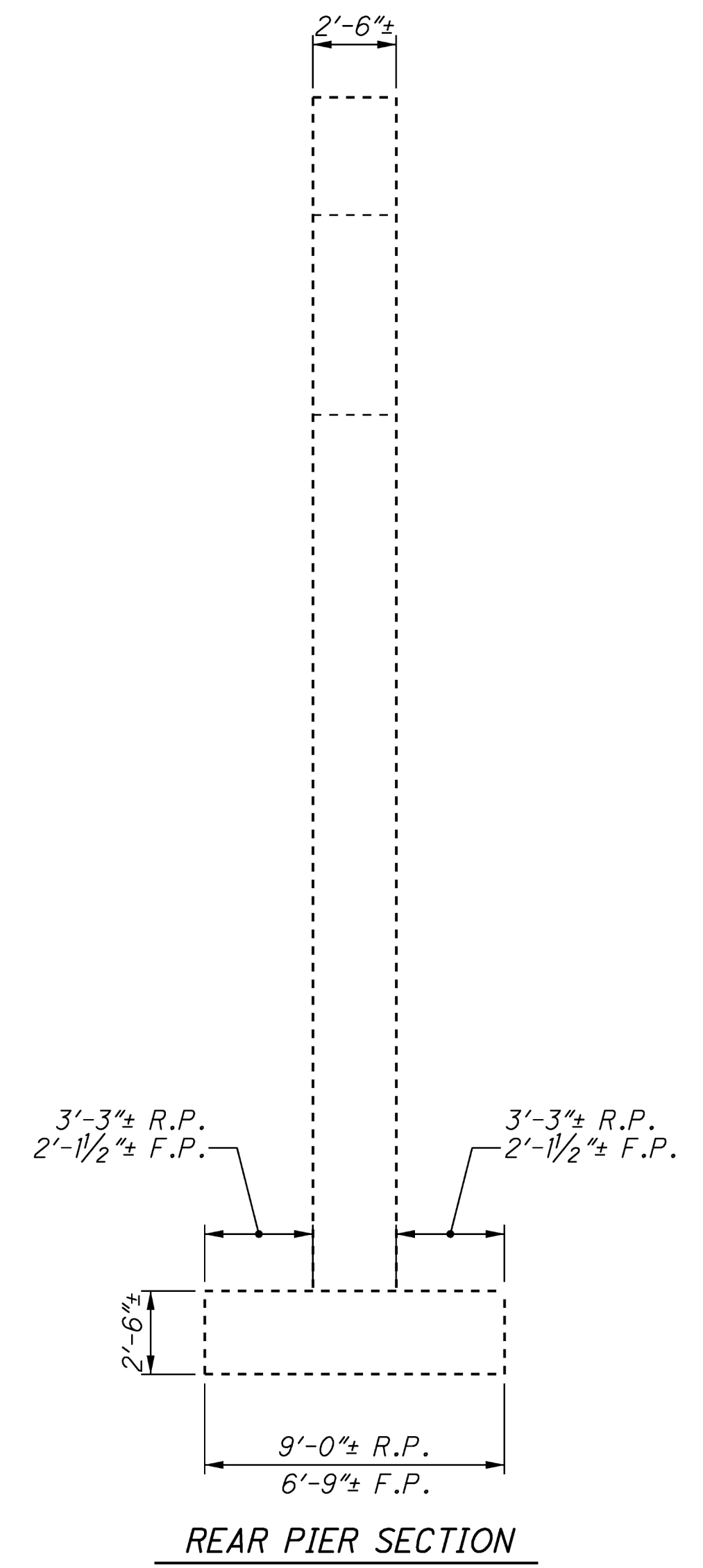
DESIGN AGENCY	O.D.O.T. DISTRICT 11
DATE	
REVIEWED	STRUCTURE FILE NUMBER 3401200
DRAWN	CCN
DESIGNED	CCN
CHECKED	RPT
REVISIONS	
ABUTMENT SECTION	BRIDGE NO. HAS-22-2126
	OVER WHEELING & LAKE ERIE RAILROAD
HAS-22-21.26	PID No. 88904
11/29	
	34
	52



PIER PLAN



PIER ELEVATION



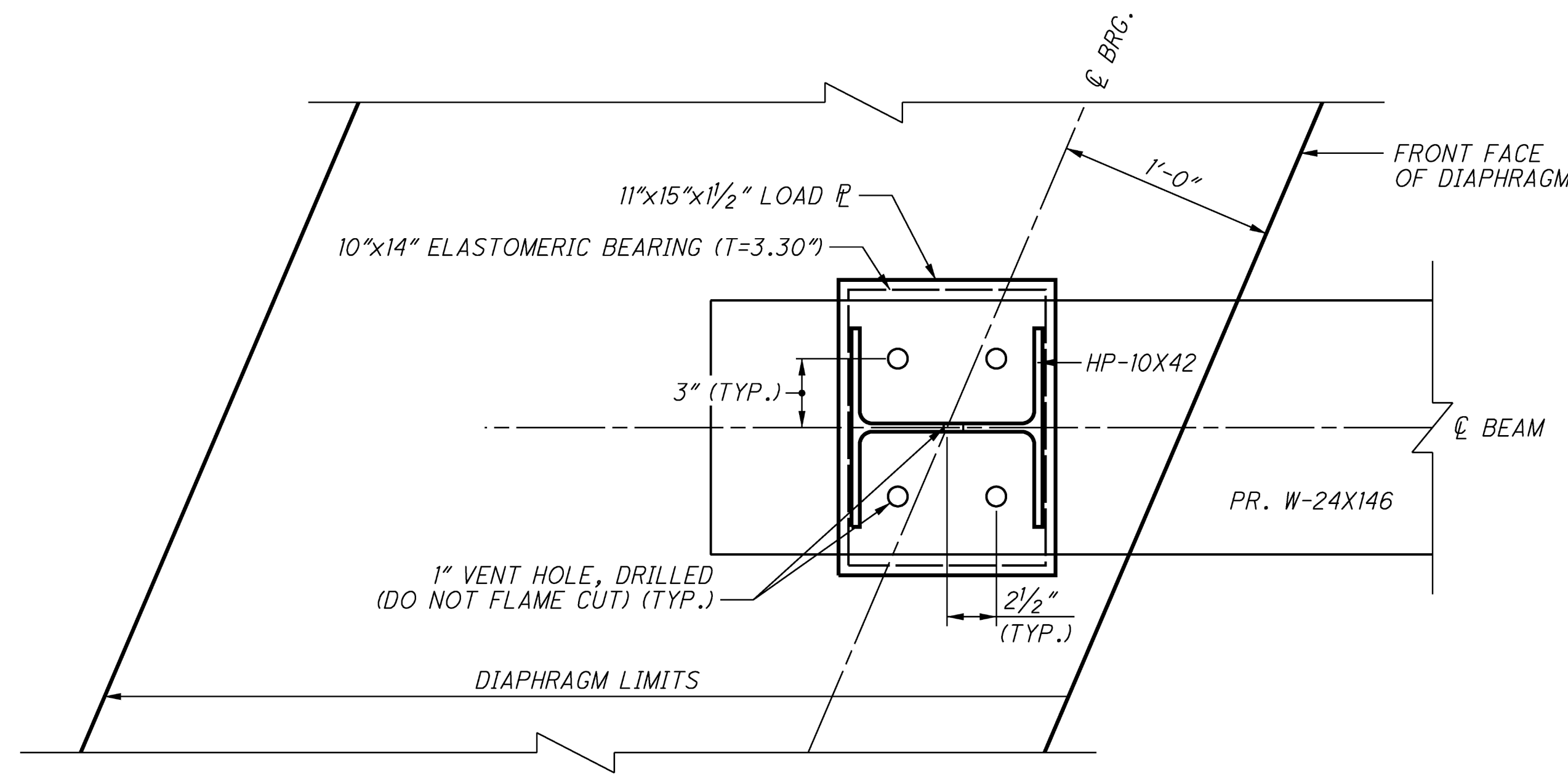
REAR PIER SECTION

NOTES

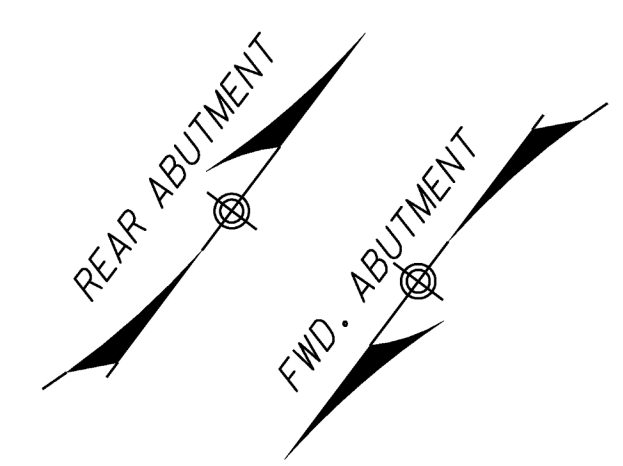
1. SEAL THE ENTIRE EXPOSED CONCRETE SURFACE OF THE PIERS, EXCEPT THE TOP OF THE CAP, AS DIRECTED BY THE REQUIREMENTS OF CMS 512.03.

ELEVATION TABLE

	A	B	C	D	E	F
REAR PIER	1147.59±	1148.23±	1148.86±	1149.47±	1150.10±	1150.00±
FORWARD PIER	1148.09±	1148.72±	1149.34±	1149.96±	1150.56±	1150.43±

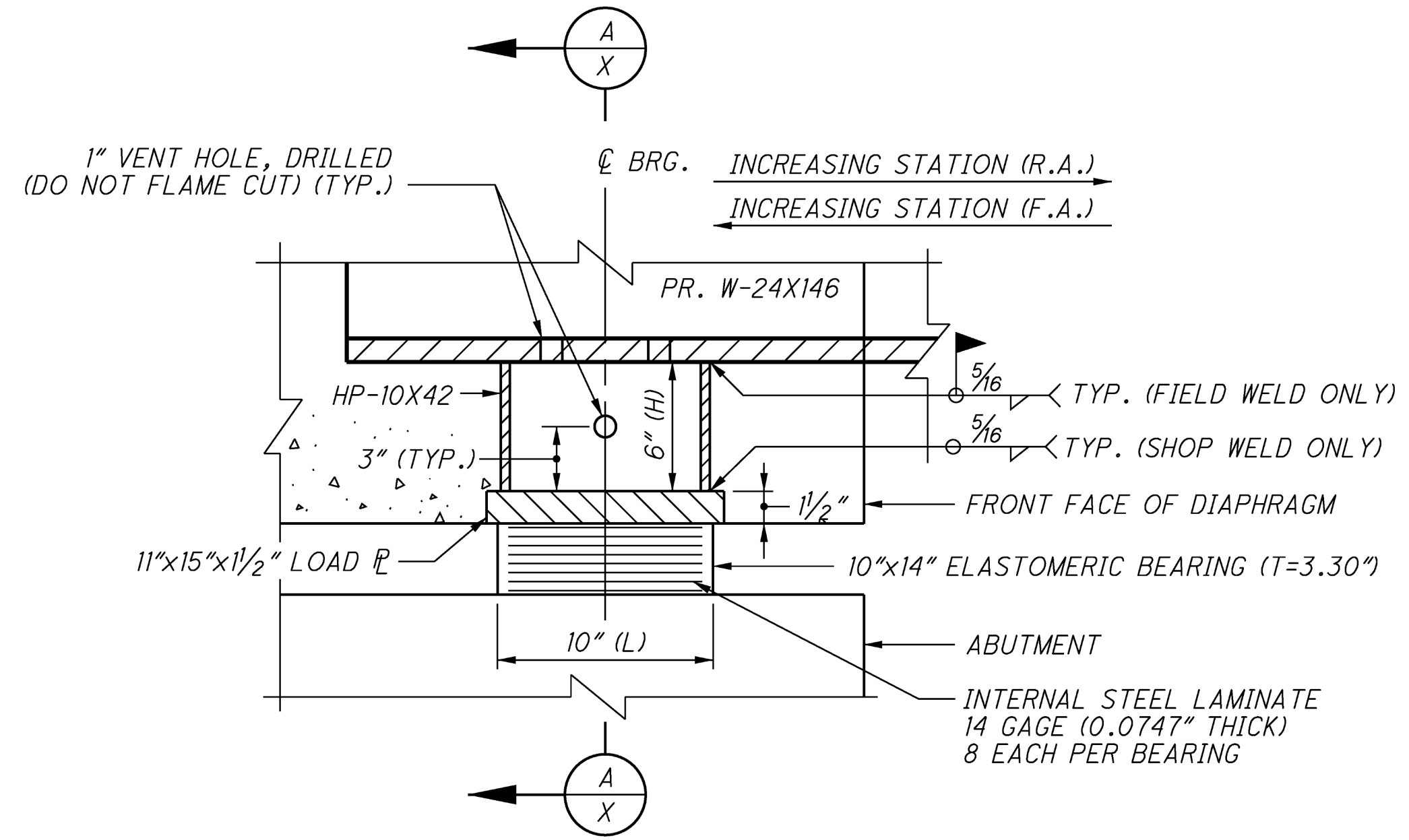


ABUTMENT BEARING PLAN VIEW
 TYP. ALL BEAMS; REAR ABUTMENT SHOWN,
 FORWARD ABUTMENT OPPOSITE HAND

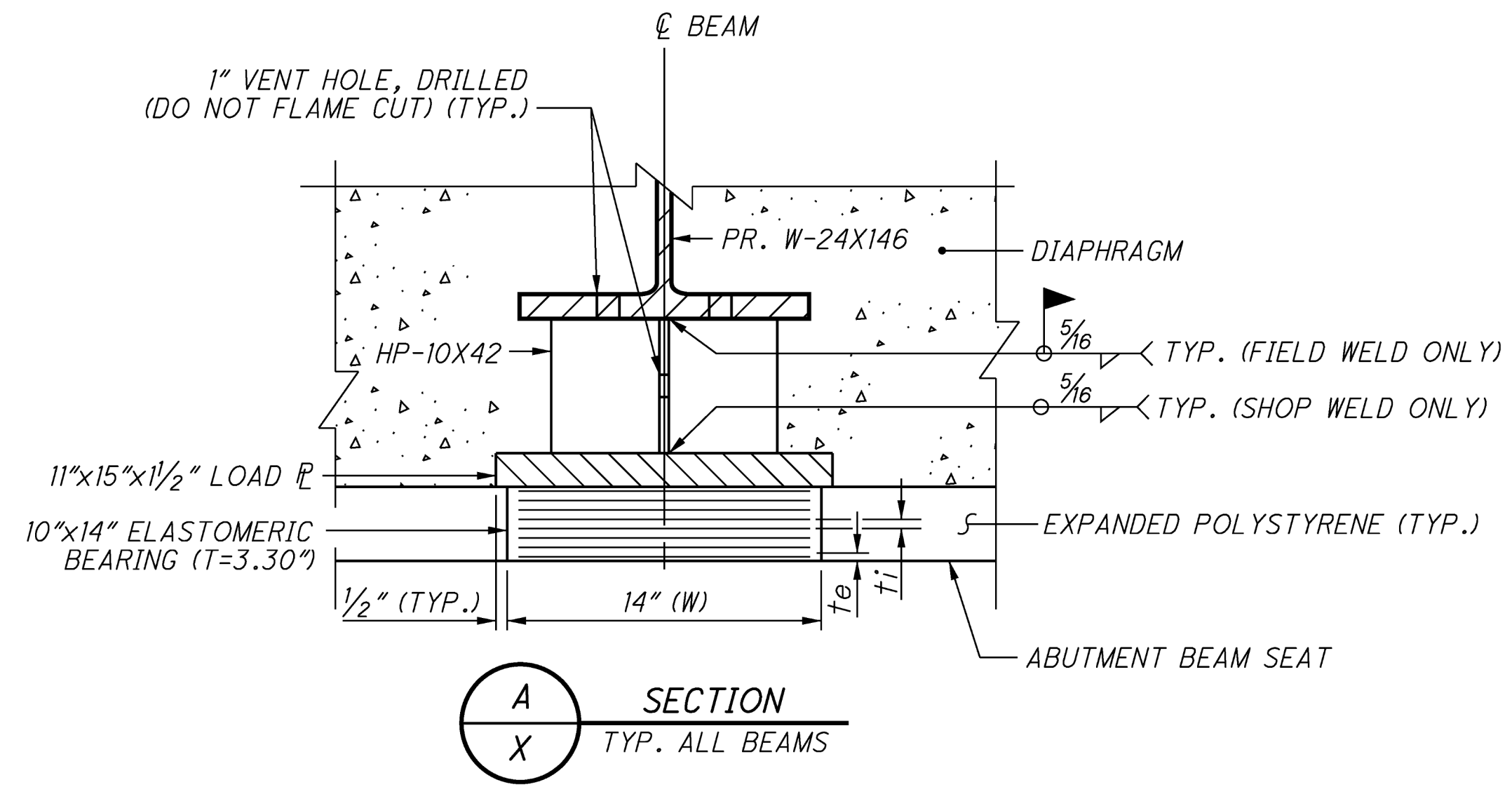


NOTES:

1. ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION I, SECTION 14.6.6 (METHOD A) OF THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.
2. ALL STEEL LOAD PLATES AND HP PEDESTALS SHALL BE THE SAME MATERIAL AS THE BEAMS. STEEL LOAD PLATES SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER DURING THE MOLDING PROCESS.
3. THE FABRICATOR SHALL PERMANENTLY MARK EACH BEARING. AT A MINIMUM, THE MARKINGS SHALL INDICATE THE SUBSTRUCTURE LOCATION, BEAM NUMBER, AND ORIENTATION TO INCREASING STATIONS. THE MARKINGS SHALL BE VISIBLE AFTER INSTALLATION.
4. BASIS OF PAYMENT: THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS, LABOR, TESTING, LOAD PLATES, PEDESTALS, AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL LAMINATED ELASTOMERIC BEARINGS. PAYMENT WILL BE MADE AT THE CONTRACT PRICE FOR THE APPROPRIATE ITEM 516, ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN.
5. FOR PIER BEARINGS, SEE SHEET 15/29.



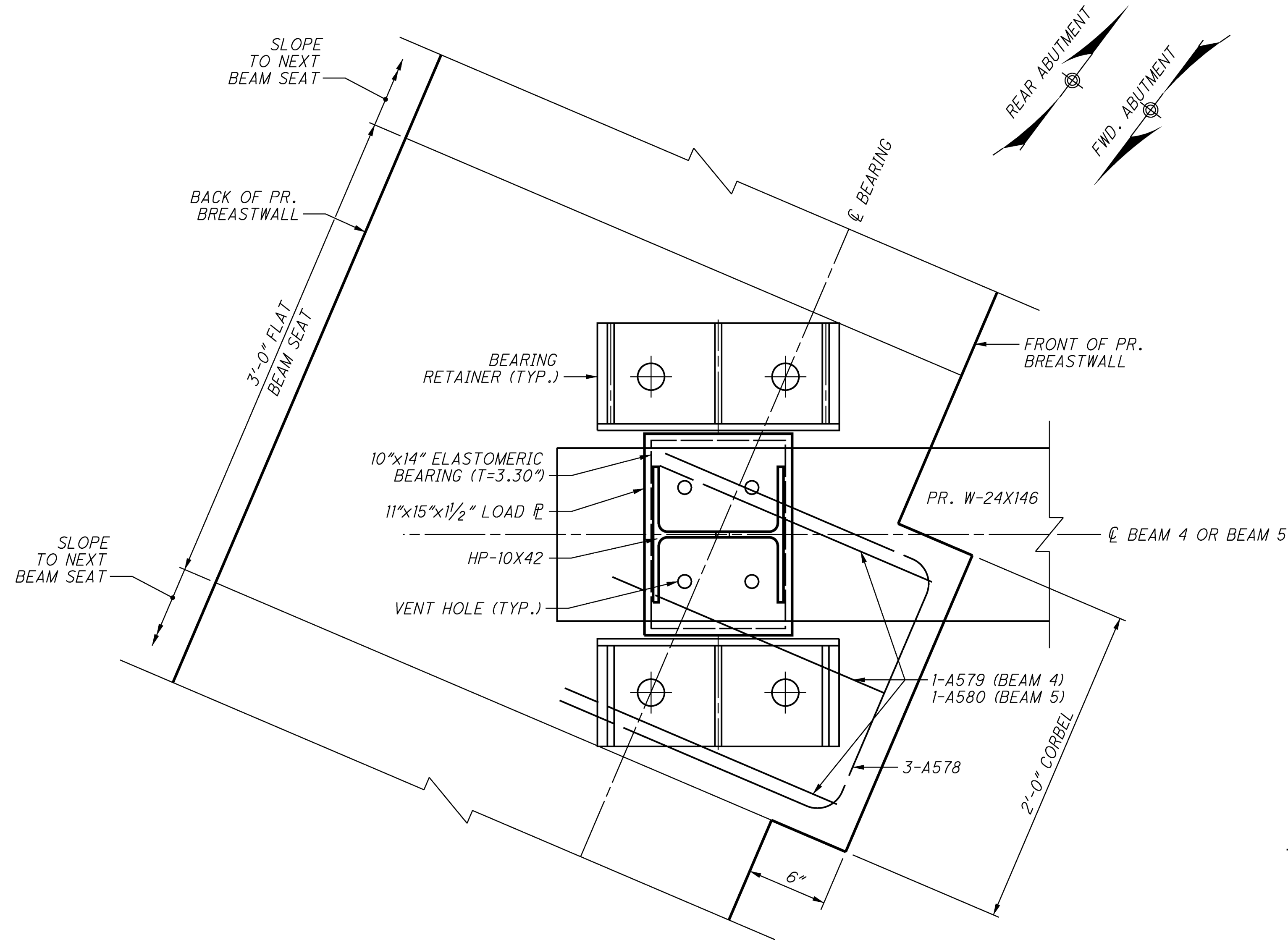
ABUTMENT BEARING ELEVATION VIEW
 TYP. ALL BEAMS; REAR ABUTMENT SHOWN,
 FORWARD ABUTMENT OPPOSITE HAND



LAMINATED ELASTOMERIC BEARINGS (ABUTMENTS)

LOCATION	BEARING DIMENSIONS						STEEL LOAD PLATE	HP-10x42 HEIGHT (H)	DEAD LOAD	LIVE LOAD WITHOUT IMPACT	MAX TOTAL LOAD
	L	W	ti	te	T	N	LENGTH X WIDTH X THICKNESS				
REAR ABUTMENT	10"	14"	0.34"	0.16"	3.30"	8	11"x15"x1-1/2"	6"	63 KIPS	49 KIPS	112 KIPS
FWD. ABUTMENT	10"	14"	0.34"	0.16"	3.30"	8	11"x15"x1-1/2"	6"	51 KIPS	46 KIPS	97 KIPS

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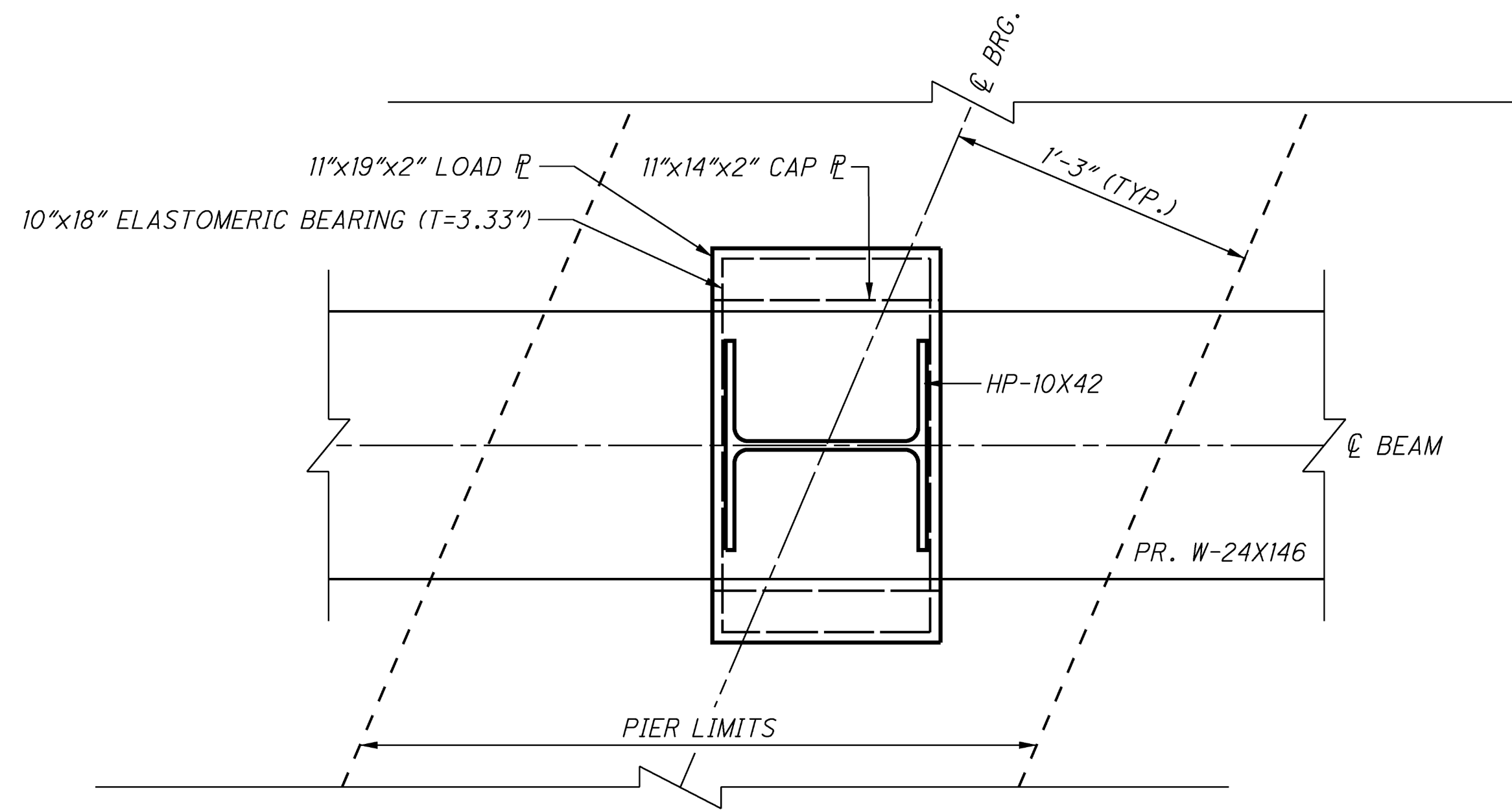
ABUTMENT BEARING PLAN VIEW

RETAINERS TYP. FOR BEAMS 4 & 5
 REAR ABUTMENT SHOWN,
 FORWARD ABUTMENT OPPOSITE HAND

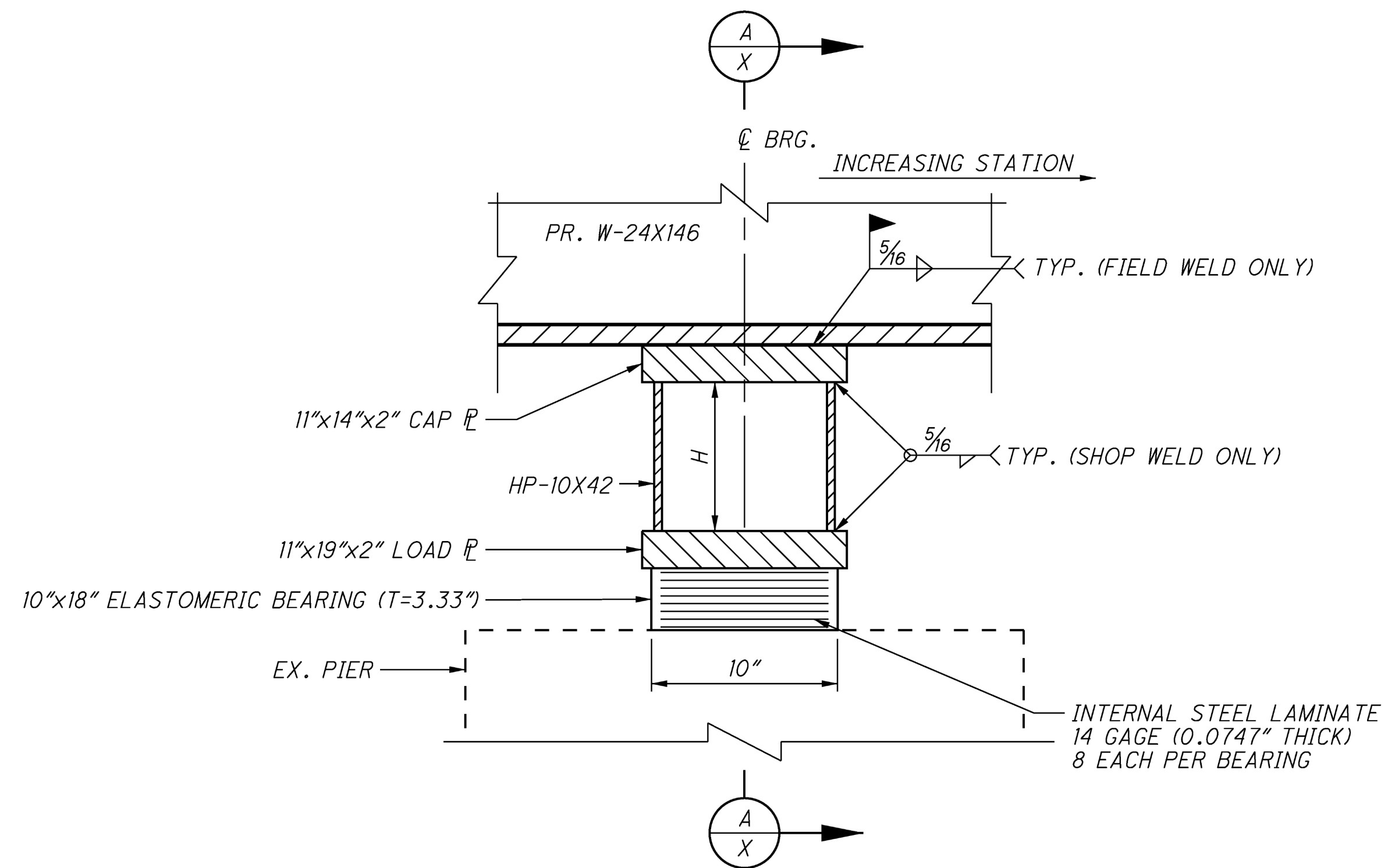
NOTES:

1. RETAINERS ARE TO BE INSTALLED ON BOTH SIDES OF BEAMS 4 & 5 ON BOTH THE REAR AND FORWARD ABUTMENTS.
2. THE COST FOR FURNISHING AND INSTALLING THE STEEL RETAINER ASSEMBLIES WILL BE INCLUDED FOR PAYMENT IN THE UNIT PRICE BID FOR THE ELASTOMERIC BEARINGS.
3. ACCURATELY PLACE REINFORCING STEEL IN THE VICINITY OF THE BRIDGE SEAT TO AVOID INTERFERENCE WITH THE DRILLING OF ANCHOR BAR HOLES.
4. FOR ELASTOMERIC BEARING DETAILS AND LOAD PLATE DIMENSIONS, SEE SHEET 13/29 .
5. FOR CORBEL LOCATIONS WITH RESPECT TO ABUTMENTS, SEE SHEETS 7/29 AND 9/29 .
6. FOR REINFORCING STEEL LIST, SEE SHEETS 26/29 - 29/29 .
7. SEE SCD SICD-1-96 FOR ADDITIONAL DETAILS.

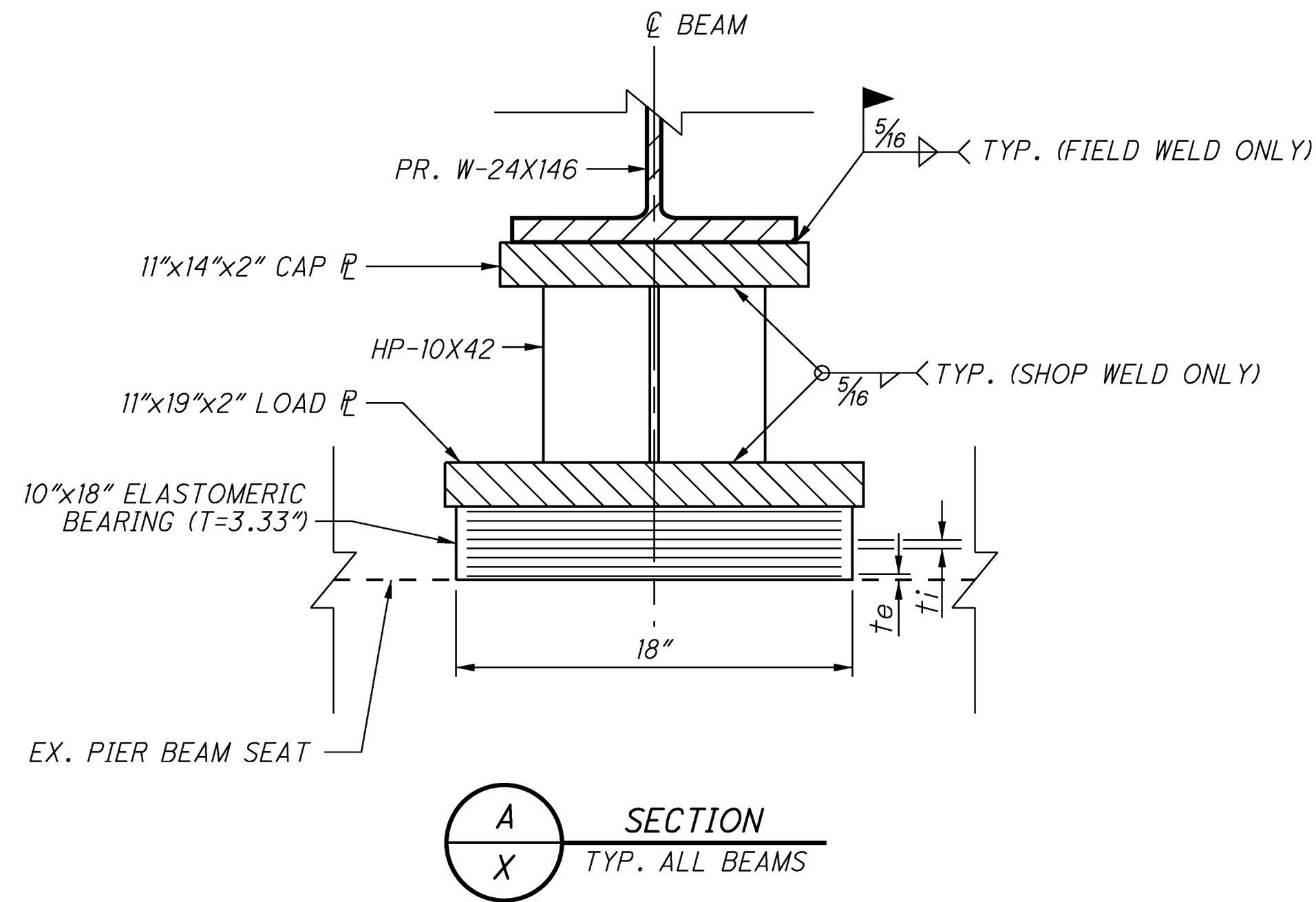
HAS-22-21.26	PID No. 88904	BEARING DETAILS (ABUTMENTS)	BRIDGE NO. HAS-22-2126 OVER WHEELING & LAKE ERIE RAILROAD	DESIGN AGENCY O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING
		DESIGNED CCN	CHECKED RPT	REVIEWED CCN
		DRAWN CCN	REVISED	DATE 3401200
		STRUCTURE FILE NUMBER 3401200		



PIER BEARING PLAN VIEW
TYP. ALL BEAMS



PIER BEARING ELEVATION VIEW
TYP. ALL BEAMS



SECTION A-X
TYP. ALL BEAMS

NOTES:

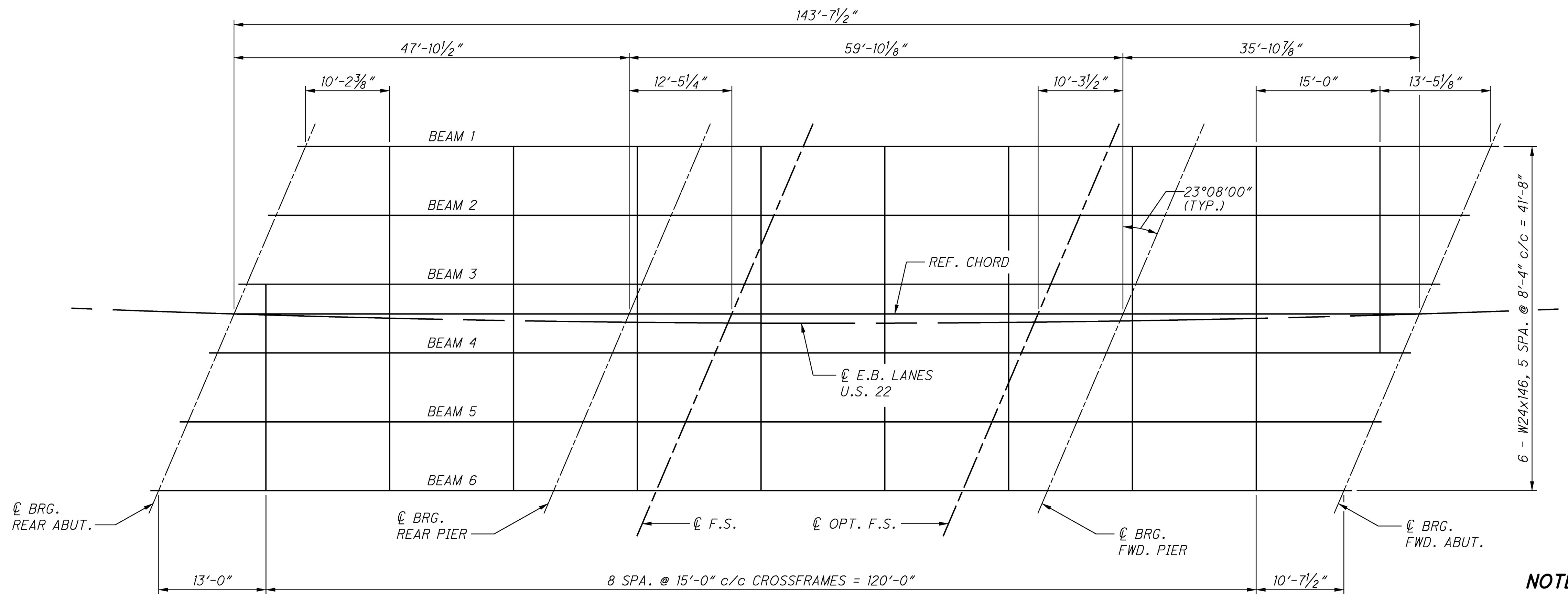
1. ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 50 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION I, SECTION 14.6.6 (METHOD A) OF THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.
2. ALL STEEL LOAD PLATES, CAP PLATES, AND HP PEDESTALS SHALL BE THE SAME MATERIAL AS THE BEAMS. STEEL LOAD PLATES SHALL BE BONDED BY VULCANIZATION TO THE ELASTOMER DURING THE MOLDING PROCESS.
3. THE FABRICATOR SHALL PERMANENTLY MARK EACH BEARING. AT A MINIMUM, THE MARKINGS SHALL INDICATE THE SUBSTRUCTURE LOCATION, BEAM NUMBER, AND ORIENTATION TO INCREASING STATIONS. THE MARKINGS SHALL BE VISIBLE AFTER INSTALLATION.
4. BASIS OF PAYMENT: THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS, LABOR, TESTING, LOAD PLATES, CAP PLATES, PEDESTALS, AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL LAMINATED ELASTOMERIC BEARINGS. PAYMENT WILL BE MADE AT THE CONTRACT PRICE FOR THE APPROPRIATE ITEM 516, ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN.
5. FOR ABUTMENT BEARINGS, SEE SHEET 13/29.

LOCATION	BEARING DIMENSIONS						STEEL LOAD PLATE		STEEL CAP PLATE		HP-10x42 HEIGHT (H)	DEAD LOAD	LIVE LOAD WITHOUT IMPACT	TOTAL
	L	W	t _i	t _e	T	N	LENGTH X WIDTH X THICKNESS	LENGTH X WIDTH X THICKNESS						
PIER 1 (REAR)	10"	18"	.33"	.21"	3.33"	8	11"x19"x2"	11"x14"x2"	VARIES, SEE HP TABLE	111 KIPS	55 KIPS	166 KIPS		
PIER 2 (FWD.)	10"	18"	.33"	.21"	3.33"	8	11"x19"x2"	11"x14"x2"		97 KIPS	57 KIPS	154 KIPS		

	HP HEIGHT (H)	
	R.P.	F.P.
BEAM 1	8 1/16"	9 5/16"
BEAM 2	8 3/8"	9 3/8"
BEAM 3	8 5/16"	9 9/16"
BEAM 4	8 3/4"	9 3/4"
BEAM 5	8 15/16"	10 3/16"
BEAM 6	9"	10 5/8"

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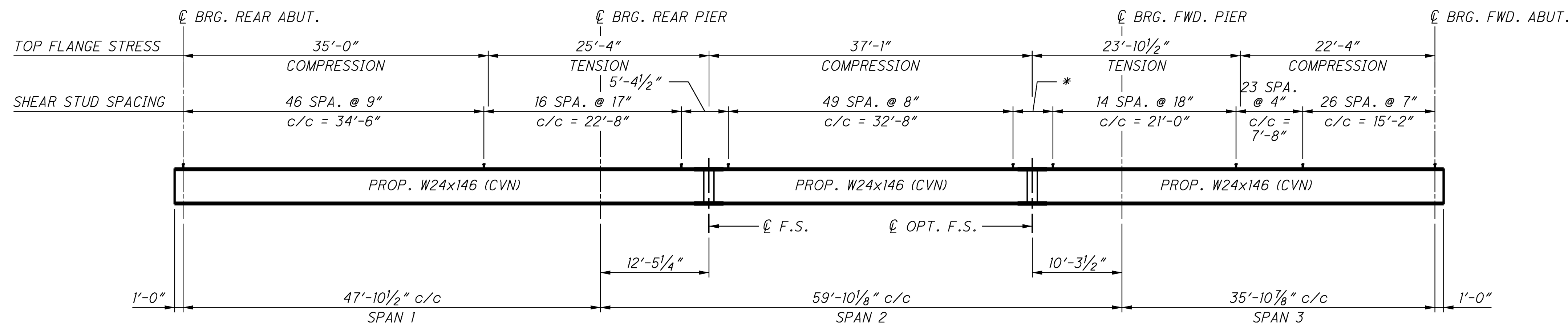
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FRAMING PLAN

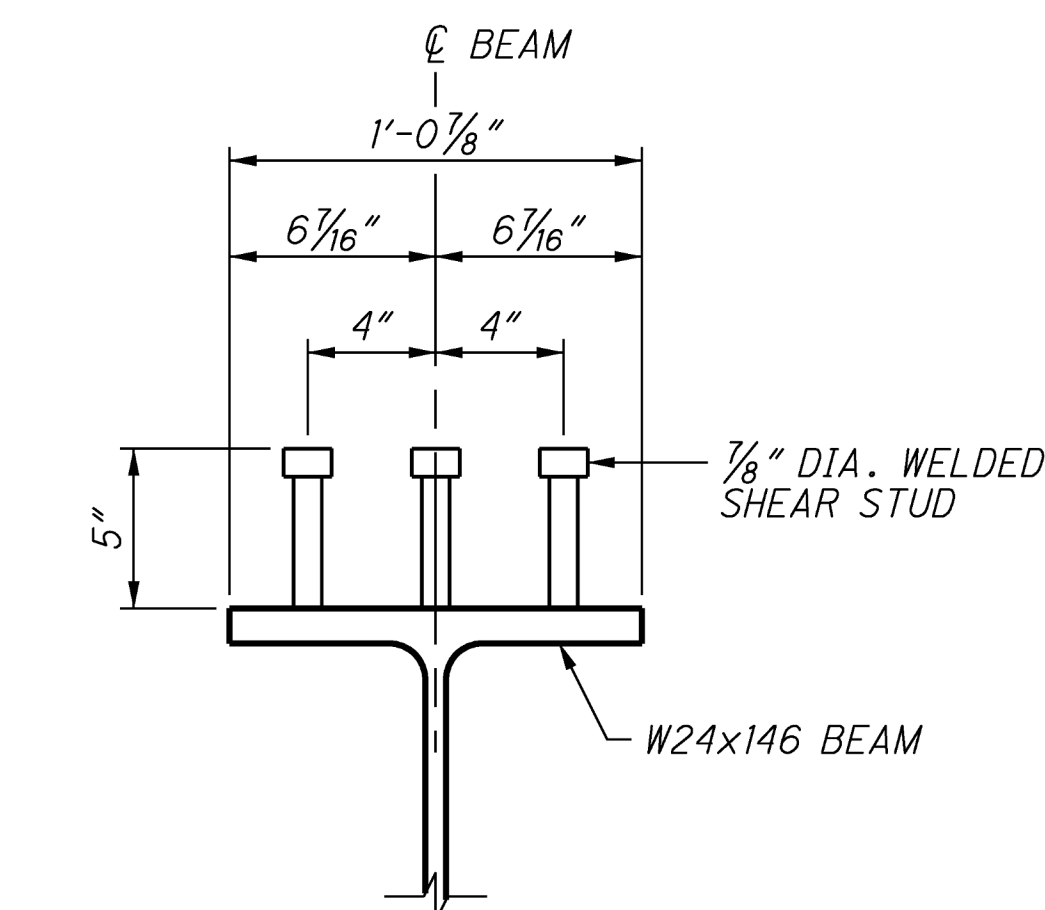
NOTES

1. WELD ATTACHMENT OF SUPPORTS FOR CONCRETE DECK FINISHING MACHINE TO AREAS OF THE FASCIA STRINGER FLANGES DESIGNATED "COMPRESSION". DO NOT WELD ATTACHMENTS TO AREAS DESIGNATED "TENSION." FILLET WELDS TO COMPRESSION FLANGES SHALL BE AT LEAST 1" FROM EDGE OF FLANGE, BE NO MORE THAN 2" LONG, AND BE AT LEAST 1/4" FOR THICKNESSES UP TO 3/4" OR 5/16" FOR GREATER THAN 3/4" THICK.
2. FOR CROSS FRAME DETAILS, SEE SHEET [20/29] AND SCD GSD-1-96.
3. FOR VENT HOLE DETAILS, SEE SHEET [13/29].

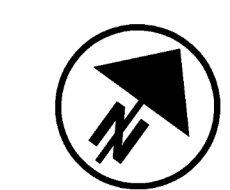


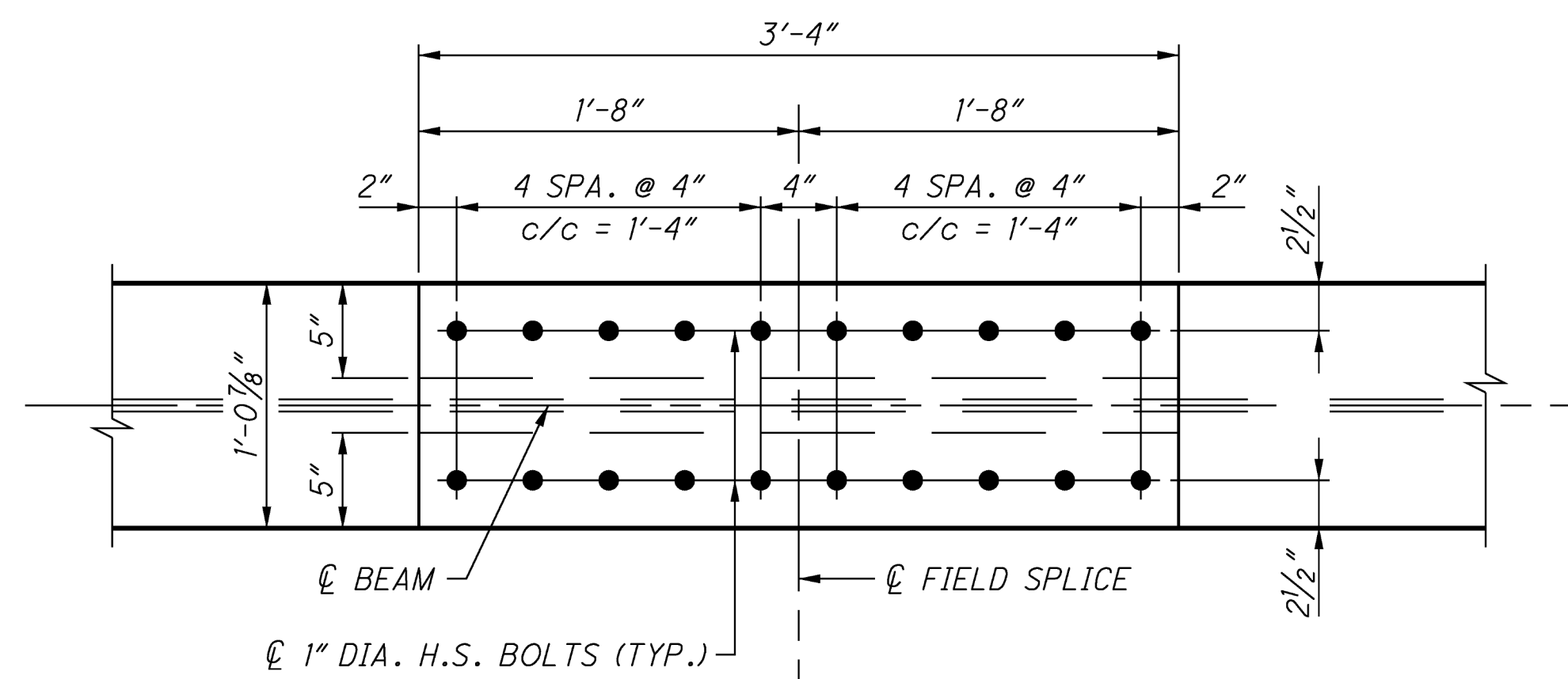
TYPICAL BEAM ELEVATION

* = 4'-7" IF OPTIONAL FIELD SPLICE IS USED
 = 6 EQUAL SPA. = 4'-7" IF OPTIONAL FIELD SPLICE IS NOT USED

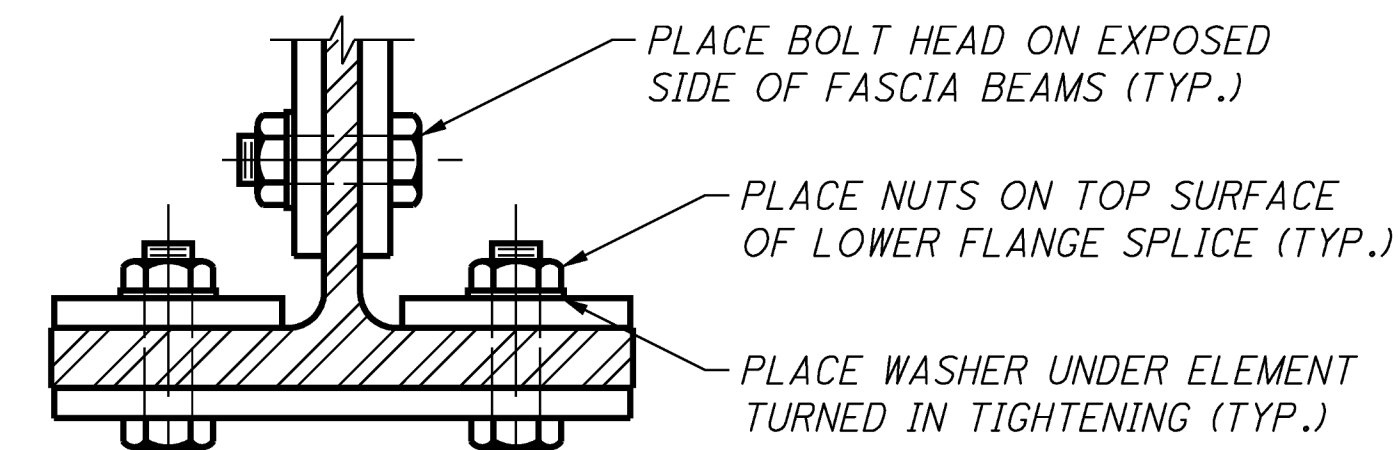


WELDED SHEAR STUD DETAIL



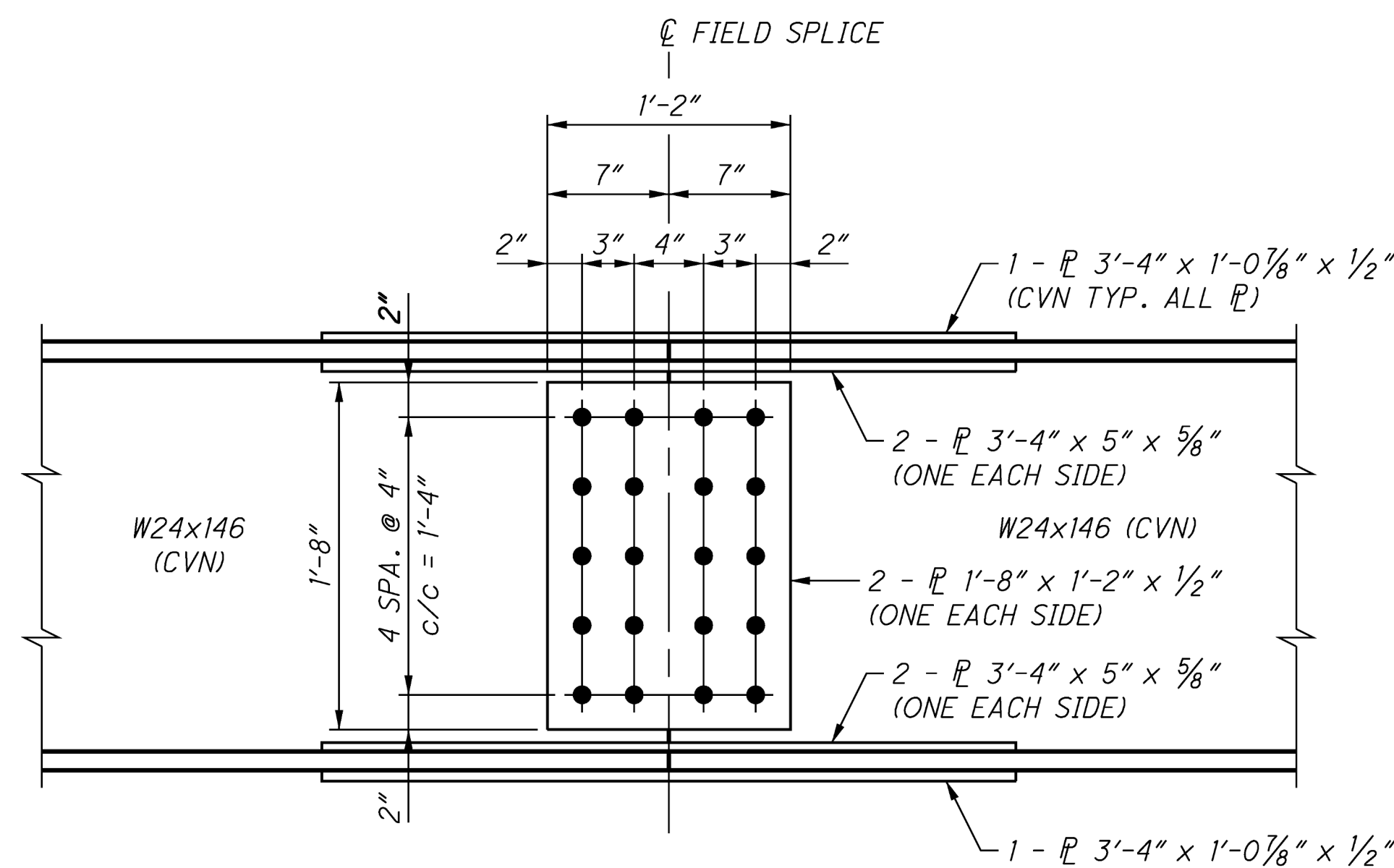


PLAN VIEW



PARTIAL SECTION

(NOT TO SCALE)



ELEVATION

BOLTED FIELD SPLICE DETAIL

NOTES

1. ALL STRUCTURAL STEEL SHALL BE ASTM A709 GRADE 50W, MINIMUM YIELD STRENGTH 50,000 PSI.
2. ALL STRUCTURAL STEEL FOR SPLICE PLATES SHALL HAVE CVN DESIGNATION AND MEET SPECIFIED MINIMUM (CVN) NOTCH TOUGHNESS REQUIREMENTS AS SPECIFIED IN 711.01.
3. HIGH STRENGTH BOLTS FOR FIELD SPLICES SHALL BE 1" DIAMETER A325 TYPE III BOLTS. HOLES FOR HIGH STRENGTH BOLTS FOR FIELD SPLICES SHALL BE 1/16" DIAMETER STANDARD HOLE SIZE.
4. BEAM ENDS AT SPLICES SHALL BE CUT AND FIT AS PER PLAN. THE OPENING BETWEEN GIRDER ENDS AFTER ASSEMBLY SHALL NOT EXCEED 1/4"
5. FOR LOCATION OF FIELD SPLICES, SEE FRAMING PLAN SHEET 16 OF 29.
6. ALL SPLICE MATERIALS, INCLUDING PLATES AND BOLT ASSEMBLIES, SHALL BE INCLUDED FOR PAYMENT UNDER ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 2.

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BEAM 1	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/4 "	1/2 "	3/4 "	15/16 "	15/16 "	15/16 "	7/8 "	7/8 "	13/16 "	13/16 "	5/8 "	7/16 "	3/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	1/4 "	1/4 "	1/8 "	0"	1/4 "	1/4 "	1/2 "	5/16 "	3/16 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	- 1/16 "	- 1/8 "	- 1/16 "	0"	- 1/8 "	- 1/8 "	- 3/16 "	- 1/8 "	- 1/8 "	0"	- 1/16 "	- 1/16 "	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	1/4 "	3/16 "	1/16 "	0"	3/16 "	3/16 "	3/8 "	1/4 "	1/8 "	0"	- 1/16 "	- 1/16 "	0"	0"

BEAM 2	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/4 "	1/2 "	3/4 "	15/16 "	15/16 "	15/16 "	7/8 "	7/8 "	13/16 "	13/16 "	5/8 "	3/8 "	3/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	5/16 "	5/16 "	1/8 "	0"	1/4 "	5/16 "	9/16 "	3/8 "	1/4 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	- 1/16 "	- 1/8 "	- 1/16 "	0"	- 1/8 "	- 1/8 "	- 3/16 "	- 1/8 "	- 1/8 "	0"	- 1/16 "	- 1/16 "	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	5/16 "	1/4 "	1/16 "	0"	3/16 "	1/4 "	7/16 "	5/16 "	3/16 "	0"	- 1/16 "	- 1/16 "	0"	0"

BEAM 3	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/4 "	1/2 "	3/4 "	15/16 "	15/16 "	15/16 "	7/8 "	7/8 "	13/16 "	13/16 "	5/8 "	3/8 "	3/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	5/16 "	5/16 "	1/8 "	0"	1/4 "	5/16 "	9/16 "	3/8 "	1/4 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	- 1/16 "	- 1/8 "	- 1/16 "	0"	- 1/8 "	- 1/8 "	- 3/16 "	- 1/8 "	- 1/8 "	0"	- 1/16 "	- 1/16 "	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	5/16 "	1/4 "	1/16 "	0"	3/16 "	1/4 "	7/16 "	5/16 "	3/16 "	0"	- 1/16 "	- 1/16 "	0"	0"

BEAM 4	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/4 "	1/2 "	11/16 "	15/16 "	15/16 "	15/16 "	7/8 "	7/8 "	13/16 "	13/16 "	5/8 "	3/8 "	3/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	5/16 "	5/16 "	1/8 "	0"	1/4 "	5/16 "	9/16 "	3/8 "	1/4 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	- 1/16 "	- 1/8 "	- 1/16 "	0"	- 1/8 "	- 1/8 "	- 3/16 "	- 1/8 "	- 1/8 "	0"	- 1/16 "	- 1/16 "	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	5/16 "	1/4 "	1/16 "	0"	3/16 "	1/4 "	7/16 "	5/16 "	3/16 "	0"	- 1/16 "	- 1/16 "	0"	0"

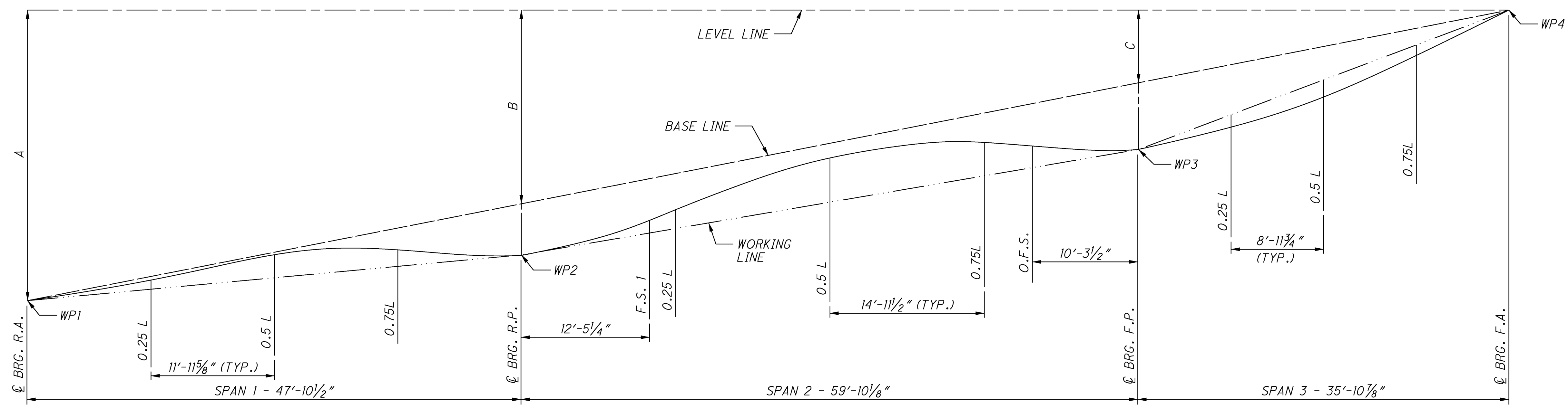
BEAM 5	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/4 "	1/2 "	11/16 "	15/16 "	15/16 "	15/16 "	7/8 "	13/16 "	13/16 "	13/16 "	5/8 "	3/8 "	3/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	5/16 "	5/16 "	1/8 "	0"	1/4 "	5/16 "	9/16 "	3/8 "	1/4 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	- 1/16 "	- 1/8 "	- 1/16 "	0"	- 1/8 "	- 1/8 "	- 3/16 "	- 1/8 "	- 1/8 "	0"	- 1/16 "	- 1/16 "	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	5/16 "	1/4 "	1/16 "	0"	3/16 "	1/4 "	7/16 "	5/16 "	3/16 "	0"	- 1/16 "	- 1/16 "	0"	0"

BEAM 6	SPAN 1						SPAN 2					SPAN 3			
	R.A.	0.25L	0.5L	0.75L	R.P.	F.S. 1	0.25L	0.5L	0.75L	O.F.S.	F.P.	0.25L	0.5L	0.75L	F.A.
DIMENSION "X"	0"	1/8 "	1/4 "	5/16 "	7/16 "	7/16 "	7/16 "	3/8 "	3/8 "	3/8 "	5/16 "	1/4 "	3/16 "	1/16 "	0"
DEFLECTION DUE TO STEEL	0"	1/16 "	1/16 "	0"	0"	1/16 "	1/16 "	1/16 "	1/16 "	1/16 "	0"	0"	0"	0"	0"
DEFLECTION DUE TO REMAINING DEAD LOAD	0"	1/4 "	1/4 "	1/8 "	0"	1/4 "	1/4 "	1/2 "	5/16 "	3/16 "	0"	0"	0"	1/16 "	0"
GEOMETRIC CAMBER	0"	1/16 "	1/16 "	1/16 "	0"	1/16 "	1/16 "	1/8 "	1/16 "	1/16 "	0"	0"	0"	- 1/16 "	0"
TOTAL SHOP CAMBER	0"	3/8 "	3/8 "	3/16 "	0"	3/8 "	3/8 "	11/16 "	7/16 "	5/16 "	0"	0"	0"	0"	0"

BLOCKING TABLE			
DIMENSION	A	B	C
BEAM 1	17 "	11 5/16 "	4 1/4 "
BEAM 2	16 11/16 "	11 1/8 "	4 3/16 "
BEAM 3	16 7/16 "	10 15/16 "	4 1/8 "
BEAM 4	16 3/16 "	10 3/4 "	4 1/16 "
BEAM 5	15 7/8 "	10 5/8 "	4 "
BEAM 6	16 9/16 "	11 1/16 "	4 1/8 "

NOTES

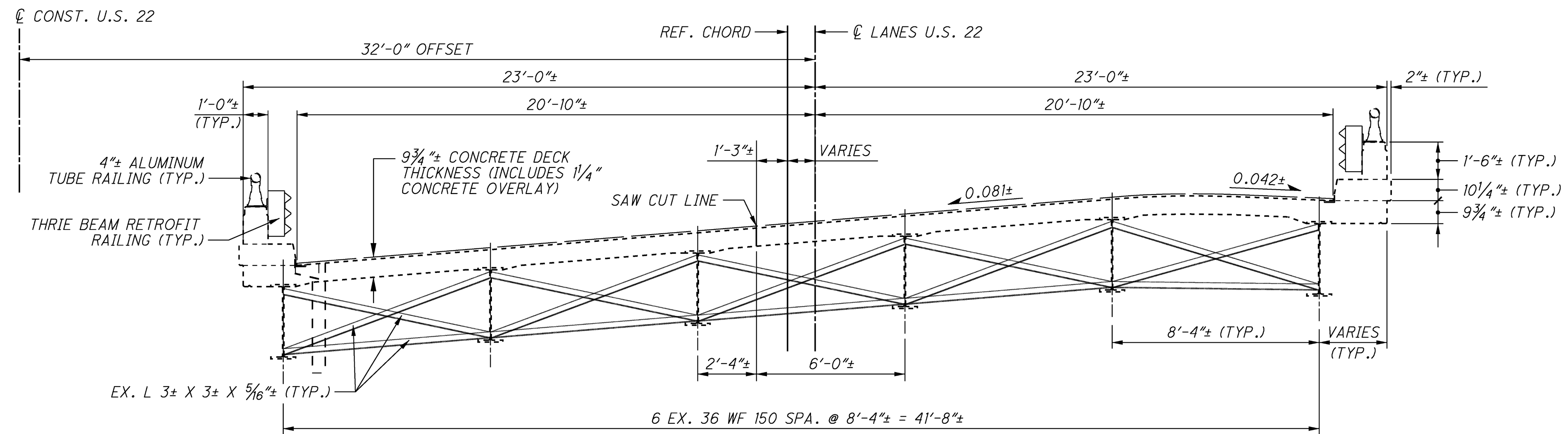
1. WORKING POINTS (WP) ARE AT THE BOTTOM OF THE BOTTOM FLANGE, ϕ BEARING.
2. THE WORKING LINE IS A STRAIGHT LINE BETWEEN THE INDICATED WORKING POINTS.
3. THE BASE LINE IS A STRAIGHT LINE BETWEEN WP1 AND WP4.
4. DIMENSION "X" IS THE DIFFERENCE BETWEEN THE BASE LINE AND THE WORKING LINE.



DEFLECTION, BLOCKING & CAMBER DIAGRAM
(BEAM SHOWN IN CORRECT UNLOADED POSITION)

DESIGNED	CCN	CHECKED	RPT
DRAWN	CCN	REVISED	
REVIEWED		STRUCTURE FILE NUMBER	3401200
DATE			
DESIGN AGENCY	O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING		
DEFLECTION AND CAMBER TABLE			
BRIDGE NO. HAS-22-2126			
OVER WHEELING & LAKE ERIE RAILROAD			
HAS-22-21.26			
PID No. 88904			
18 / 29			
41			
52			

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NOTES:

1. REMOVE ENTIRE SUPERSTRUCTURE.

HAS-22-21.26
PID No. 88904

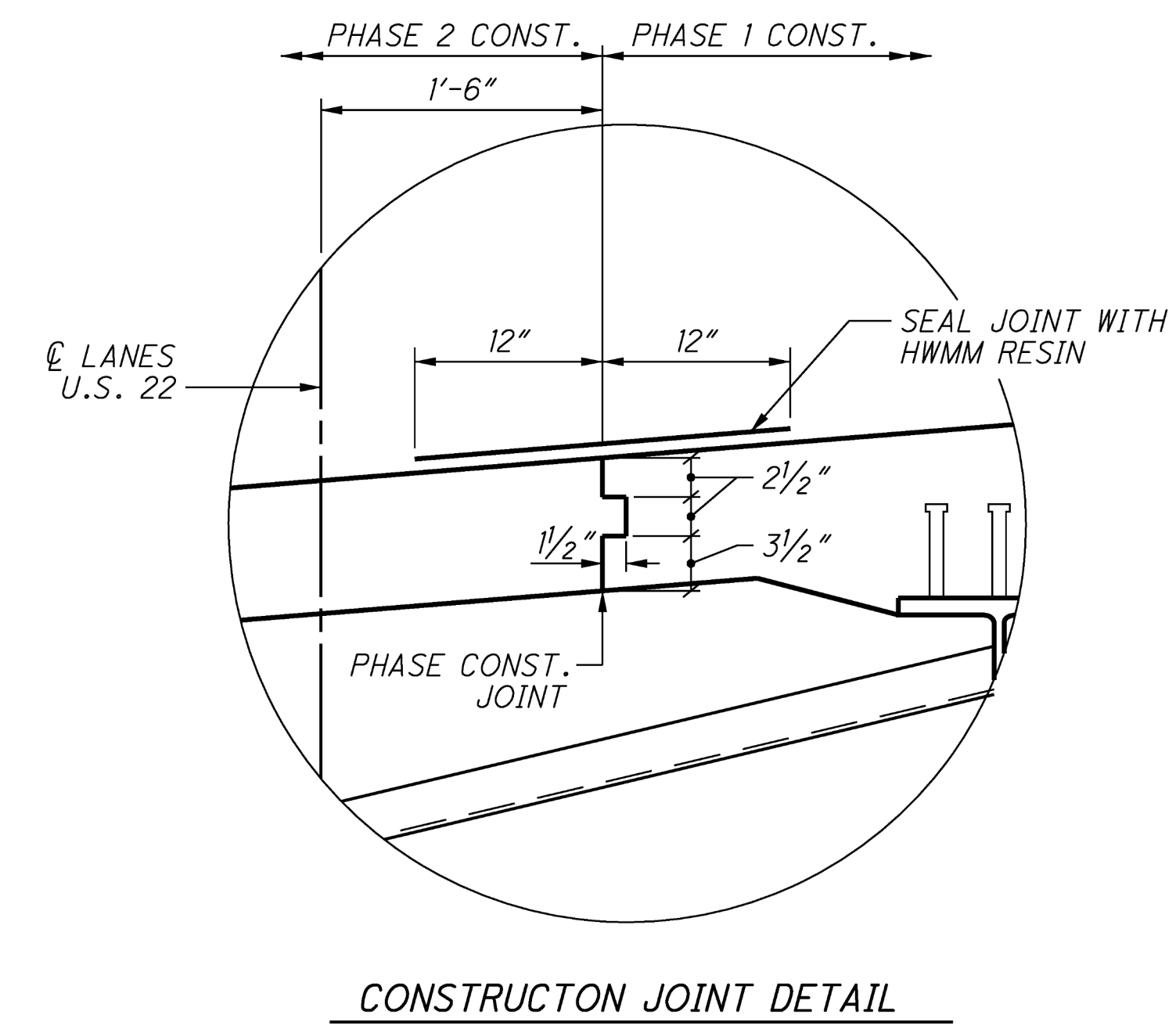
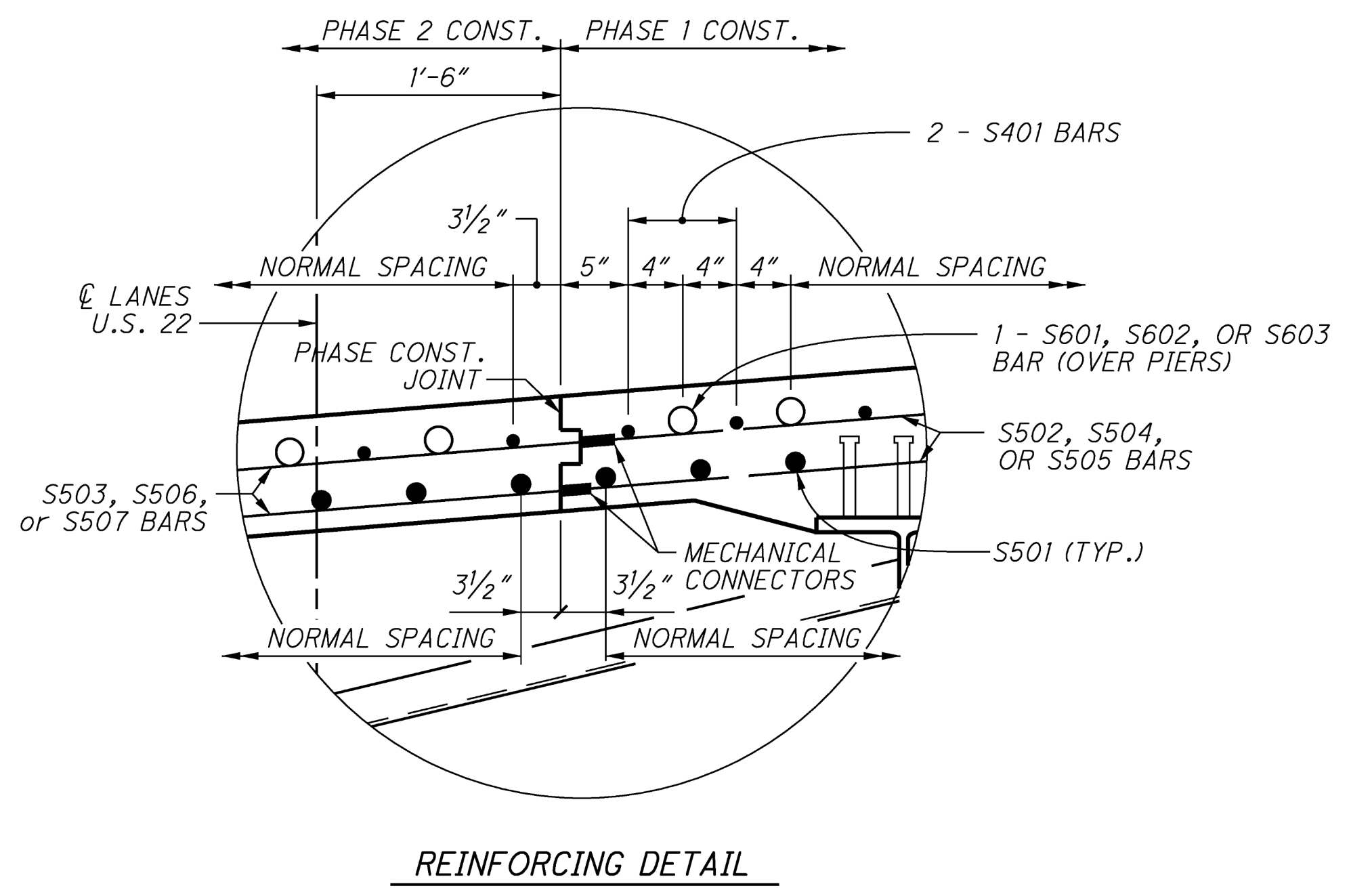
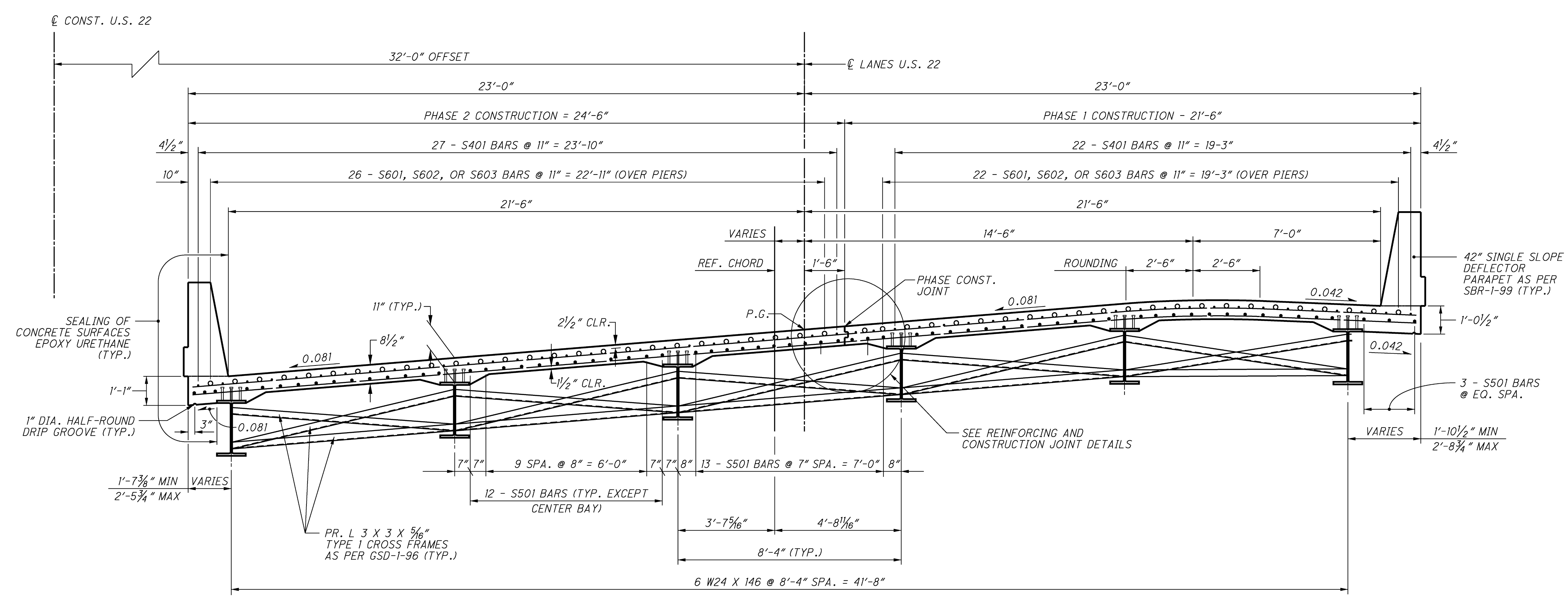
EXISTING TRANSVERSE SECTION
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

DESIGNED
CCN
CHECKED
RPT

DRAWN
CCN
REVISED

REVIEWED
STRUCTURE FILE NUMBER
3401200

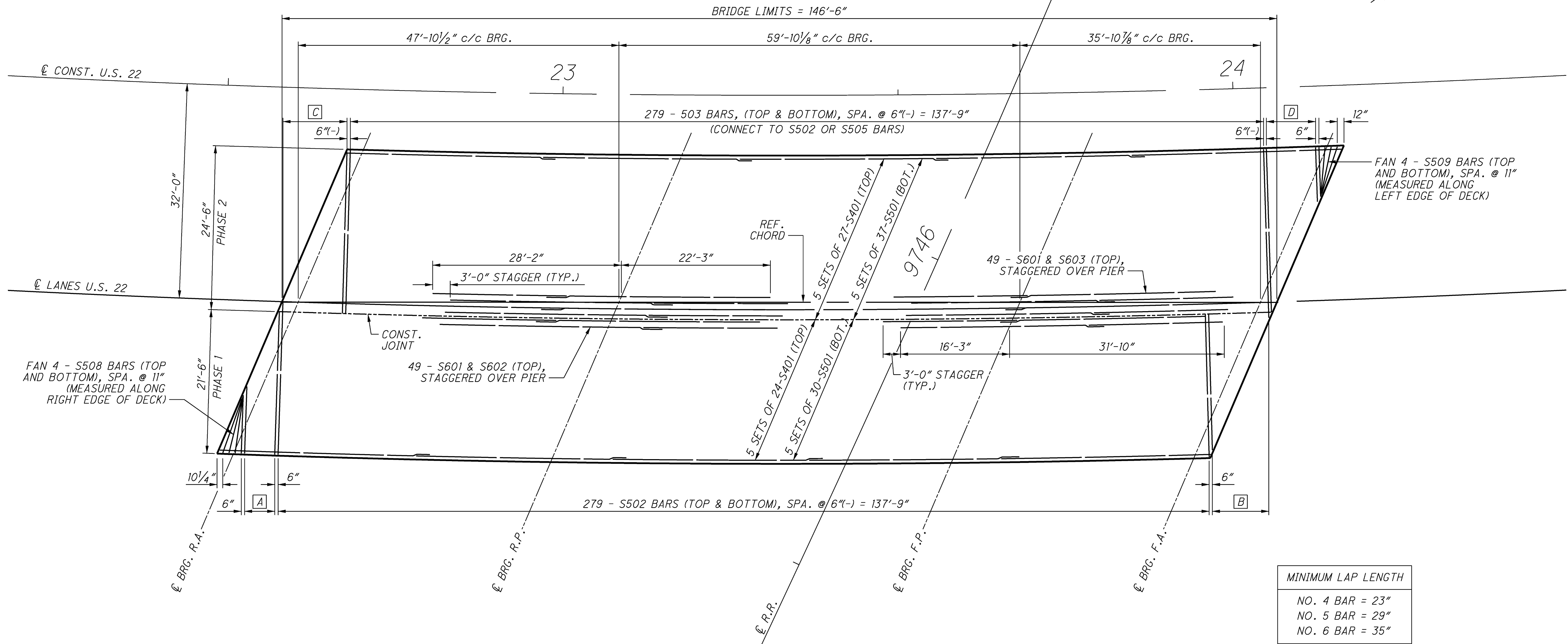
DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING



- NOTES:**
- DECK SLAB CONCRETE QUANTITY: THE ESTIMATED QUANTITY OF DECK SLAB CONCRETE IS BASED ON CONSTANT DECK SLAB THICKNESS, AS SHOWN, PLUS THE QUANTITY OF CONCRETE THAT FORMS EACH BEAM HAUNCH. THE ESTIMATE ASSUMES A CONSTANT HAUNCH THICKNESS OF 2 1/2" AND A CONSTANT HAUNCH FLANGE WIDTH OF 9". DEVIATE FROM THIS HAUNCH THICKNESS AS NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. THE ALLOWABLE TOLERANCE FOR THE HAUNCH WIDTH OUTSIDE THE EDGE OF EACH BEAM FLANGE IS ± 3".
 - TOP LONGITUDINAL BARS SHOULD BE PARALLEL TO THE EDGE OF THE DECK. BOTTOM BARS SHOULD BE PARALLEL TO BEAMS EXCEPT FOR THE BARS ADJACENT TO THE PHASE CONSTRUCTION JOINT, WHICH SHOULD BE PARALLEL TO THE JOINT, AND THE BARS CLOSEST TO THE EDGE OF DECK, WHICH SHOULD BE PARALLEL TO THE EDGE OF DECK.
 - FOR REINFORCING STEEL LIST, SEE SHEETS [26/29] - [29/29].
 - FOR BRIDGE RAILING DETAILS, SEE SHEET [25/29].
 - FOR DECK SLAB DETAILS, SEE SHEET [21/29].
 - FOR FRAMING PLAN, SEE SHEET [16/29].

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- [A] 1 SERIES OF 10 - S504 BARS (TOP AND BOTTOM), SPA. @ 6" = 4'-6" (MEASURED ALONG RIGHT EDGE OF DECK)
- [B] 1 SERIES OF 19 - S505 BARS (TOP AND BOTTOM), SPA. @ 6"(-) = 8'-11"
- [C] 1 SERIES OF 19 - S506 BARS (TOP AND BOTTOM), SPA. @ 6"(-) = 8'-11" (CONNECT TO S502 BARS)
- [D] 1 SERIES OF 16 - S507 BARS (TOP AND BOTTOM), SPA. @ 6"(-) = 7'-4" (MEASURED ALONG LEFT EDGE OF DECK)

NOTES:

1. TOP LONGITUDINAL BARS SHOULD BE PARALLEL TO THE EDGE OF THE DECK. BOTTOM BARS SHOULD BE PARALLEL TO BEAMS EXCEPT FOR THE BARS ADJACENT TO THE PHASE CONSTRUCTION JOINT AND THE EDGE OF DECK, WHICH SHOULD BE PARALLEL TO THEIR RESPECTIVE EDGE OF CONCRETE.
2. UNLESS OTHERWISE NOTED, ALL TRANSVERSE BAR SPACING IS MEASURED ALONG THE CONSTRUCTION JOINT.
3. FOR REINFORCING STEEL LIST, SEE SHEETS 26/29 - 29/29.
4. FOR BRIDGE RAILING DETAILS, SEE SHEET 25/29.
5. FOR TRANSVERSE SECTION DETAILS, SEE SHEET 20/29.
6. FOR FRAMING PLAN, SEE SHEET 16/29.

DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING

DATE
REVIEWED
STRUCTURE FILE NUMBER
3401200

DRAWN
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RPT

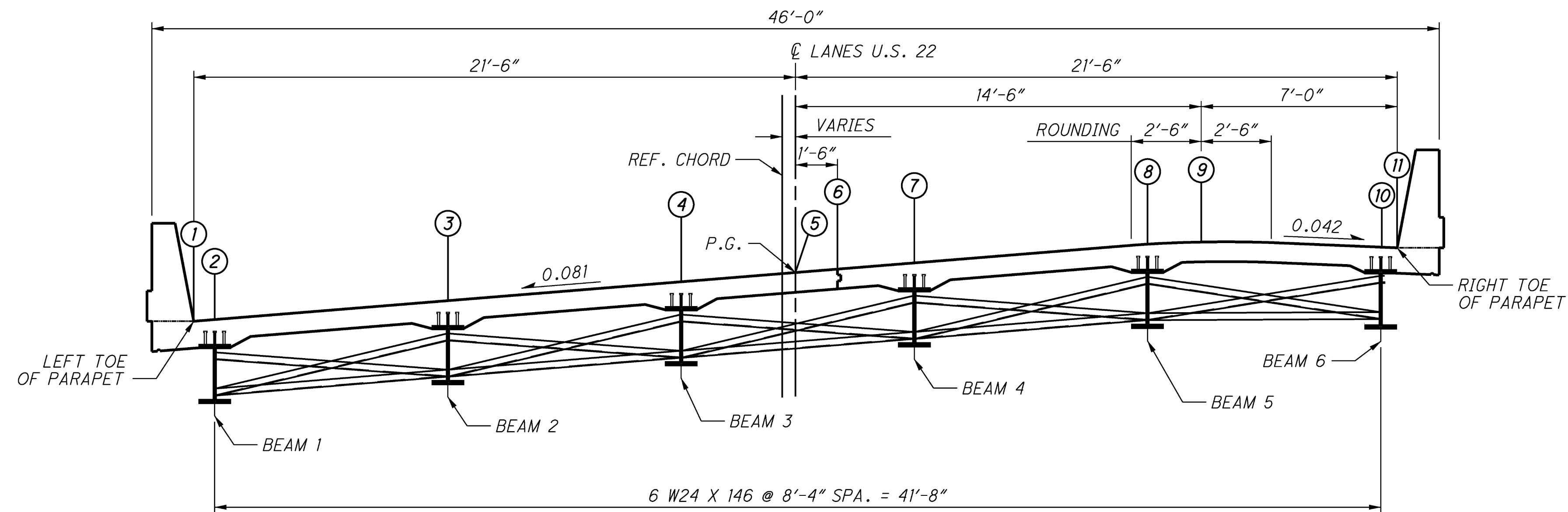
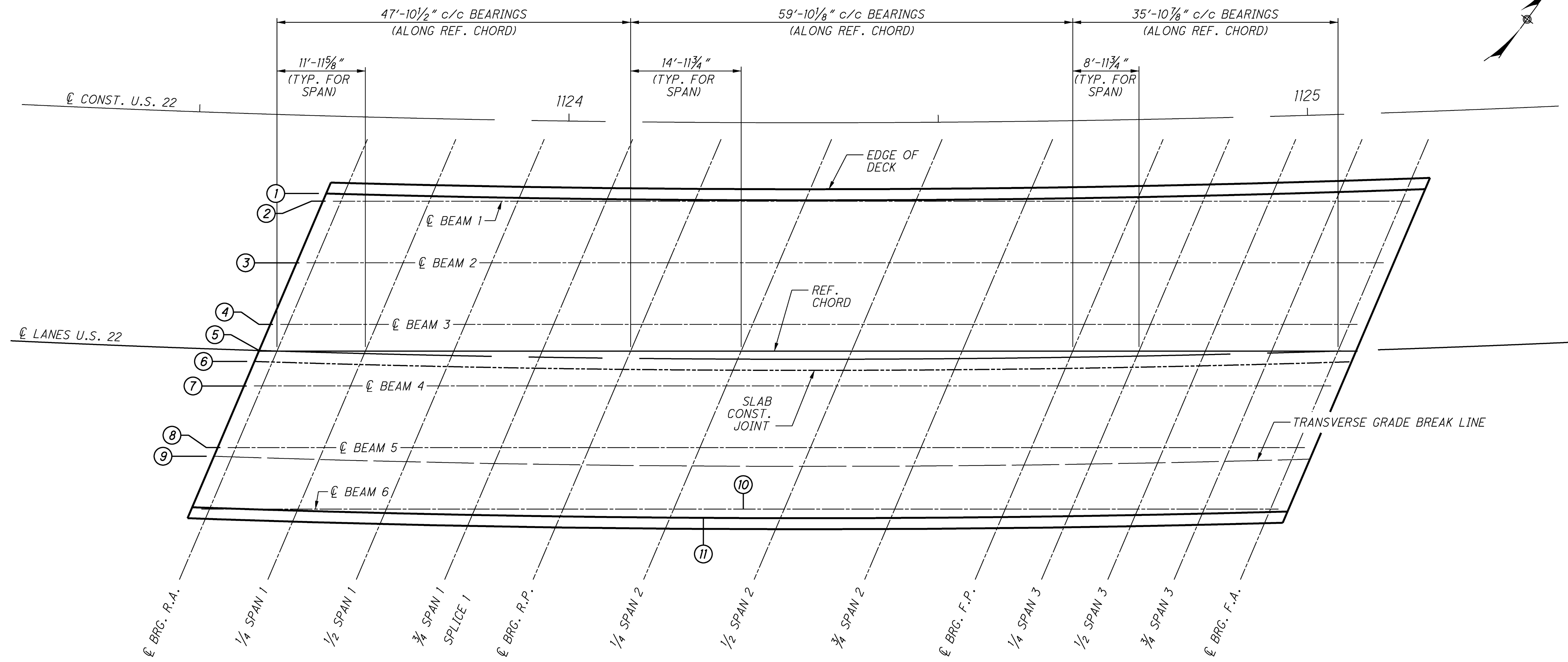
SLAB PLAN
BRIDGE NO. HAS-22-21.26
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
PID No. 88904

21/29

44
52

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- NOTES:
- FOR FINAL DECK SURFACE ELEVATIONS, SEE SHEET [24/29]. FOR SCREED AND TOP OF HAUNCH ELEVATIONS, SEE SHEET [23/29].

DESIGN AGENCY O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING	DATE 3401200	REVIEWED CCN	DESIGNED CCN	TABLE ELEVATIONS REFERENCE BRIDGE NO. HAS-22-21.26 OVER WHEELING & LAKE ERIE RAILROAD
STRUCTURE FILE NUMBER 3401200	CHECKED	REVISED	HAS-22-21.26 PID No. 88904	22/29
				45 52

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SCREED ELEVATIONS														
	SPAN 1								SPAN 2					
	CL BRG. R.A.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. R.P.		F.S. No. 1		1/4 SPAN	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
LEFT CURB	1123+69.84	1151.38	1123+81.63	1151.52	1123+93.45	1151.64	1124+05.29	1151.73	1124+17.16	1151.84	1124+29.52	1151.98	1124+32.03	1152.01
PROFILE GRADE LINE	1123+61.46	1153.04	1123+73.13	1153.19	1123+84.82	1153.30	1123+96.53	1153.39	1124+08.27	1153.49	1124+20.50	1153.63	1124+22.98	1153.67
CONSTRUCTION JOINT	1123+60.88	1153.16	1123+72.54	1153.30	1123+84.22	1153.41	1123+95.93	1153.51	1124+07.66	1153.61	1124+19.88	1153.75	1124+22.36	1153.78
GRADE BREAK	1123+55.90	1154.17	1123+67.49	1154.31	1123+79.09	1154.42	1123+90.73	1154.51	1124+02.38	1154.61	1124+14.52	1154.75	1124+16.98	1154.78
RIGHT CURB	1123+53.25	1153.85	1123+64.79	1153.98	1123+76.36	1154.10	1123+87.95	1154.19	1123+99.56	1154.29	1124+11.66	1154.43	1124+14.11	1154.46

SCREED ELEVATIONS																
	SPAN 2								SPAN 3							
	1/2 SPAN		3/4 SPAN		OPTIONAL F.S.		CL BRG. F.P.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. F.A.	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
LEFT CURB	1124+46.94	1152.17	1124+61.90	1152.30	1124+66.58	1152.33	1124+76.90	1152.41	1124+85.92	1152.50	1124+94.95	1152.58	1125+04.01	1152.67	1125+13.08	1152.76
PROFILE GRADE LINE	1124+37.73	1153.84	1124+52.52	1153.96	1124+57.16	1153.98	1124+67.36	1154.06	1124+76.28	1154.15	1124+85.21	1154.23	1124+94.16	1154.32	1125+03.13	1154.40
CONSTRUCTION JOINT	1124+37.10	1153.95	1124+51.88	1154.07	1124+56.50	1154.10	1124+66.70	1154.18	1124+75.61	1154.26	1124+84.54	1154.35	1124+93.48	1154.43	1125+02.44	1154.52
GRADE BREAK	1124+31.62	1154.95	1124+46.31	1155.07	1124+50.90	1155.10	1124+61.03	1155.17	1124+69.88	1155.26	1124+78.75	1155.34	1124+87.64	1155.43	1124+96.54	1155.52
RIGHT CURB	1124+28.70	1154.62	1124+43.33	1154.74	1124+47.92	1154.77	1124+58.01	1154.85	1124+66.83	1154.94	1124+75.66	1155.02	1124+84.52	1155.11	1124+93.39	1155.19

TOP OF HAUNCH ELEVATIONS														
	SPAN 1								SPAN 2					
	CL BRG. R.A.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. R.P.		F.S. No. 1		1/4 SPAN	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
BEAM 1	1123+69.45	1150.75	1123+81.36	1150.86	1123+93.27	1150.96	1124+05.18	1151.04	1124+17.09	1151.14	1124+29.47	1151.28	1124+31.98	1151.31
BEAM 2	1123+66.15	1151.40	1123+78.01	1151.52	1123+89.88	1151.61	1124+01.75	1151.69	1124+13.62	1151.78	1124+25.95	1151.92	1124+28.45	1151.95
BEAM 3	1123+62.87	1152.05	1123+74.69	1152.17	1123+86.51	1152.26	1123+98.34	1152.34	1124+10.16	1152.43	1124+22.45	1152.57	1124+24.95	1152.60
BEAM 4	1123+59.62	1152.70	1123+71.39	1152.82	1123+83.17	1152.91	1123+94.95	1152.98	1124+06.74	1153.07	1124+18.98	1153.20	1124+21.47	1153.24
BEAM 5	1123+56.39	1153.36	1123+68.12	1153.47	1123+79.86	1153.56	1123+91.60	1153.63	1124+03.34	1153.72	1124+15.54	1153.85	1124+18.01	1153.88
BEAM 6	1123+53.18	1153.13	1123+64.87	1153.28	1123+76.56	1153.41	1123+88.26	1153.52	1123+99.96	1153.63	1124+12.12	1153.77	1124+14.58	1153.80

TOP OF HAUNCH ELEVATIONS																
	SPAN 2								SPAN 3							
	1/2 SPAN		3/4 SPAN		OPTIONAL F.S.		CL BRG. F.P.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. F.A.	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
BEAM 1	1124+46.87	1151.48	1124+61.77	1151.61	1124+66.42	1151.65	1124+76.65	1151.74	1124+85.58	1151.84	1124+94.51	1151.95	1125+03.44	1152.05	1125+12.37	1152.16
BEAM 2	1124+43.29	1152.12	1124+58.13	1152.26	1124+62.77	1152.29	1124+72.96	1152.38	1124+81.86	1152.48	1124+90.76	1152.58	1124+99.66	1152.68	1125+08.55	1152.79
BEAM 3	1124+39.73	1152.77	1124+54.52	1152.89	1124+59.14	1152.93	1124+69.30	1153.01	1124+78.17	1153.11	1124+87.03	1153.21	1124+95.90	1153.31	1125+04.76	1153.42
BEAM 4	1124+36.20	1153.39	1124+50.93	1153.52	1124+55.54	1153.56	1124+65.66	1153.65	1124+74.50	1153.74	1124+83.34	1153.84	1124+92.17	1153.95	1125+01.00	1154.05
BEAM 5	1124+32.69	1154.05	1124+47.37	1154.17	1124+51.96	1154.20	1124+62.05	1154.28	1124+70.86	1154.38	1124+79.66	1154.48	1124+88.46	1154.58	1124+97.27	1154.68
BEAM 6	1124+29.21	1153.97	1124+43.84	1154.09	1124+48.41	1154.12	1124+58.47	1154.19	1124+67.24	1154.27	1124+76.01	1154.35	1124+84.79	1154.42	1124+93.56	1154.51

NOTES:

- SCREED ELEVATIONS SHOWN REPRESENT THE THEORETICAL DECK SURFACE LOCATION PRIOR TO DEFLECTIONS CAUSED BY DECK PLACEMENT AND OTHER ANTICIPATED DEAD LOADS.
- TOP OF HAUNCH ELEVATIONS SHOWN REPRESENT THE THEORETICAL LOCATION OF THE TOP OF HAUNCH PRIOR TO DEFLECTIONS CAUSED BY DECK PLACEMENT AND OTHER ANTICIPATED DEAD LOADS.
- SEE SHEET [22/29] FOR PLAN AND TRANSVERSE SECTION REFERENCES TO THE ELEVATIONS SHOWN IN THESE TABLES.

DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING

REVIEWED
DATE
STRUCTURE FILE NUMBER
3401200

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SCREED AND TOP OF HAUNCH ELEVATIONS
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
PID No. 88904

23/29

46
52

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FINAL DECK SURFACE ELEVATIONS														
	SPAN 1								SPAN 2					
	CL BRG. R.A.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. R.P.		F.S. No. 1		1/4 SPAN	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
LEFT CURB	1123+69.84	1151.38	1123+81.63	1151.50	1123+93.45	1151.61	1124+05.29	1151.72	1124+17.16	1151.84	1124+29.52	1151.96	1124+32.03	1151.98
BEAM 1	1123+69.45	1151.46	1123+81.36	1151.55	1123+93.27	1151.64	1124+05.18	1151.74	1124+17.09	1151.85	1124+29.47	1151.97	1124+31.98	1151.99
BEAM 2	1123+66.15	1152.11	1123+78.01	1152.20	1123+89.88	1152.29	1124+01.75	1152.39	1124+13.62	1152.49	1124+25.95	1152.61	1124+28.45	1152.63
BEAM 3	1123+62.87	1152.76	1123+74.69	1152.85	1123+86.51	1152.94	1123+98.34	1153.04	1124+10.16	1153.14	1124+22.45	1153.25	1124+24.95	1153.27
CONSTRUCTION JOINT	1123+60.88	1153.16	1123+72.54	1153.27	1123+84.22	1153.38	1123+95.93	1153.50	1124+07.66	1153.61	1124+19.88	1153.73	1124+22.36	1153.75
BEAM 4	1123+59.62	1153.41	1123+71.39	1153.50	1123+83.17	1153.59	1123+94.95	1153.68	1124+06.74	1153.78	1124+18.98	1153.89	1124+21.47	1153.92
BEAM 5	1123+56.39	1154.07	1123+68.12	1154.15	1123+79.86	1154.24	1123+91.60	1154.33	1124+03.34	1154.43	1124+15.54	1154.54	1124+18.01	1154.56
GRADE BREAK	1123+55.90	1154.17	1123+67.49	1154.28	1123+79.09	1154.39	1123+90.73	1154.50	1124+02.38	1154.61	1124+14.52	1154.73	1124+16.98	1154.75
BEAM 6	1123+53.18	1153.84	1123+64.87	1153.97	1123+76.56	1154.09	1123+88.26	1154.22	1123+99.96	1154.34	1124+12.12	1154.46	1124+14.58	1154.48
RIGHT CURB	1123+53.25	1153.85	1123+64.79	1153.96	1123+76.36	1154.07	1123+87.95	1154.18	1123+99.56	1154.29	1124+11.66	1154.41	1124+14.11	1154.43

FINAL DECK SURFACE ELEVATIONS																
	SPAN 2						SPAN 3									
	1/2 SPAN		3/4 SPAN		OPTIONAL F.S.		CL BRG. F.P.		1/4 SPAN		1/2 SPAN		3/4 SPAN		CL BRG. F.A.	
	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION	STATION	ELEVATION
LEFT CURB	1124+46.94	1152.12	1124+61.90	1152.27	1124+66.58	1152.31	1124+76.90	1152.41	1124+85.92	1152.50	1124+94.95	1152.58	1125+04.01	1152.67	1125+13.08	1152.76
BEAM 1	1124+46.87	1152.14	1124+61.77	1152.29	1124+66.42	1152.34	1124+76.65	1152.45	1124+85.58	1152.55	1124+94.51	1152.66	1125+03.44	1152.76	1125+12.37	1152.87
BEAM 2	1124+43.29	1152.77	1124+58.13	1152.93	1124+62.77	1152.98	1124+72.96	1153.09	1124+81.86	1153.19	1124+90.76	1153.29	1124+99.66	1153.39	1125+08.55	1153.50
BEAM 3	1124+39.73	1153.41	1124+54.52	1153.56	1124+59.14	1153.61	1124+69.30	1153.72	1124+78.17	1153.82	1124+87.03	1153.92	1124+95.90	1154.02	1125+04.76	1154.13
CONSTRUCTION JOINT	1124+37.10	1153.89	1124+51.88	1154.03	1124+56.50	1154.08	1124+66.70	1154.18	1124+75.61	1154.26	1124+84.54	1154.35	1124+93.48	1154.43	1125+02.44	1154.52
BEAM 4	1124+36.20	1154.05	1124+50.93	1154.20	1124+55.54	1154.25	1124+65.66	1154.36	1124+74.50	1154.45	1124+83.34	1154.55	1124+92.17	1154.66	1125+01.00	1154.76
BEAM 5	1124+32.69	1154.70	1124+47.37	1154.84	1124+51.96	1154.89	1124+62.05	1154.99	1124+70.86	1155.09	1124+79.66	1155.19	1124+88.46	1155.29	1124+97.27	1155.39
GRADE BREAK	1124+31.62	1154.89	1124+46.31	1155.03	1124+50.90	1155.08	1124+61.03	1155.17	1124+69.88	1155.26	1124+78.75	1155.34	1124+87.64	1155.43	1124+96.54	1155.52
BEAM 6	1124+29.21	1154.63	1124+43.84	1154.77	1124+48.41	1154.81	1124+58.47	1154.90	1124+67.24	1154.98	1124+76.01	1155.06	1124+84.79	1155.13	1124+93.56	1155.22
RIGHT CURB	1124+28.70	1154.57	1124+43.33	1154.71	1124+47.92	1154.75	1124+58.01	1154.85	1124+66.83	1154.94	1124+75.66	1155.02	1124+84.52	1155.11	1124+93.39	1155.19

NOTES:

1. FINAL DECK SURFACE ELEVATIONS SHOWN REPRESENT THE DECK SURFACE LOCATION AFTER ALL ANTICIPATED DEAD LOAD DEFLECTIONS HAVE OCCURRED.
2. SEE SHEET [22/29] FOR PLAN AND TRANSVERSE SECTION REFERENCES TO THE ELEVATIONS SHOWN IN THIS TABLE.

DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING

REVIEWED DATE
STRUCTURE FILE NUMBER
3401200

DRAWN CCN
CHECKED DJL
DESIGNED CCN
REVISED

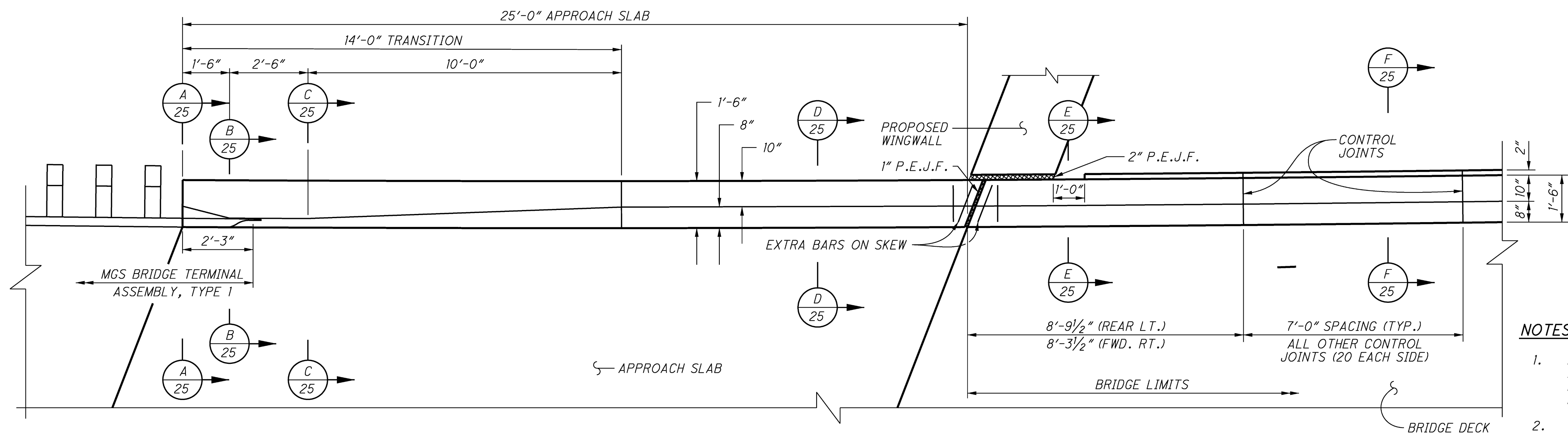
FINAL DECK SURFACE ELEVATIONS
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
PID No. 88904

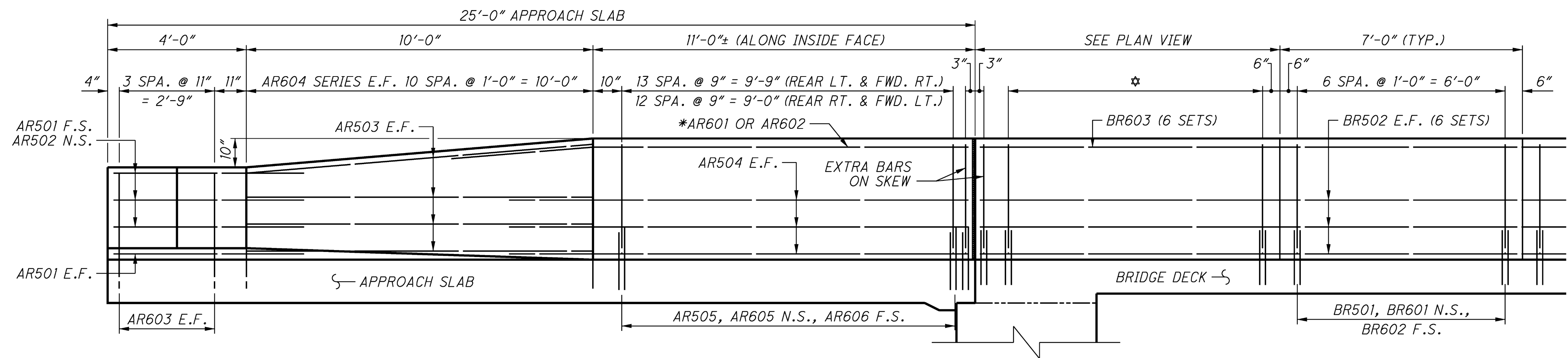
24/29

47
52

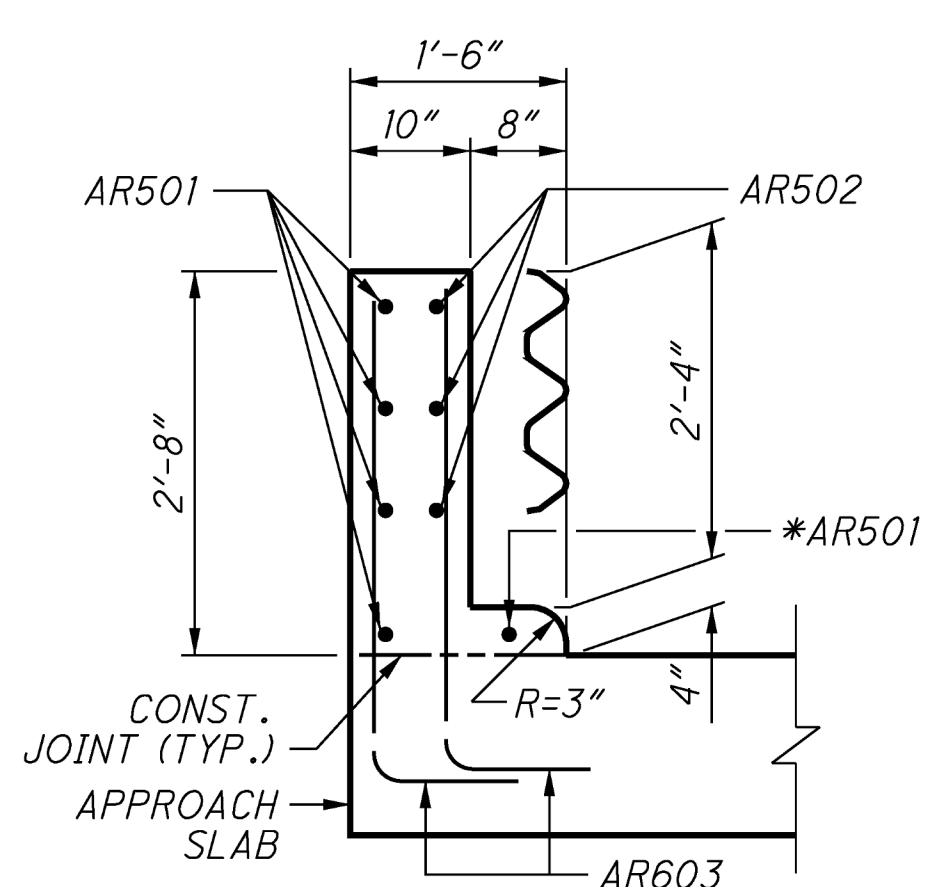
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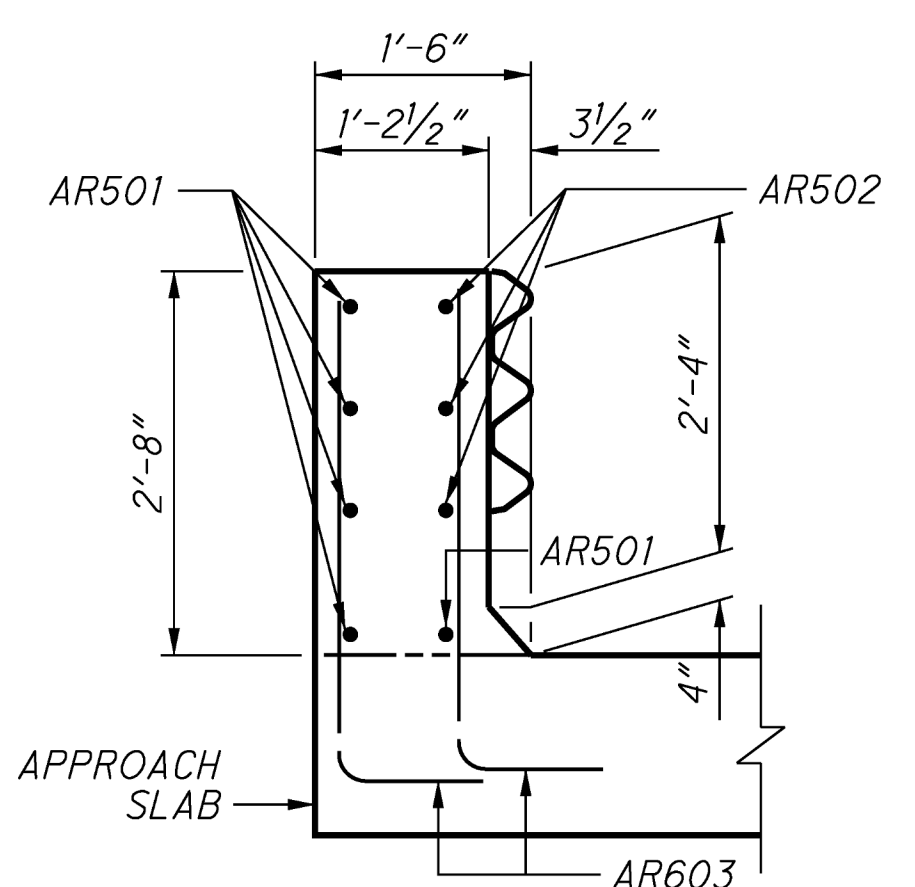
DEFLECTOR PARAPET PLAN
LEFT REAR SHOWN, OTHERS SIMILAR



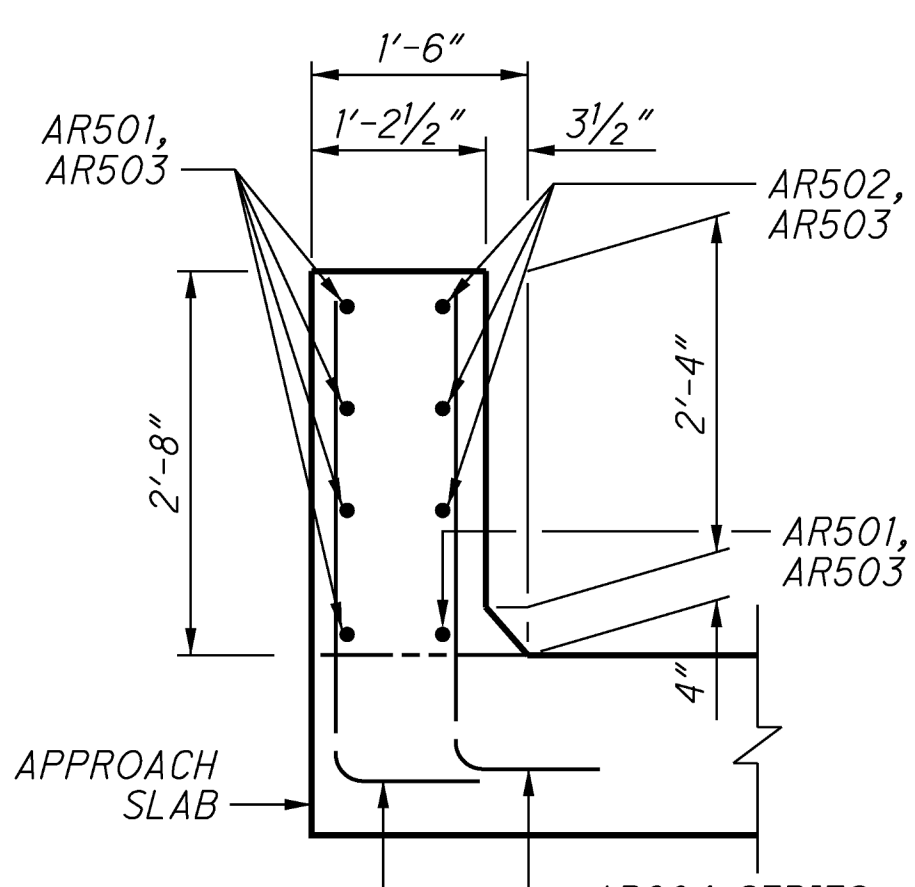
DEFLECTOR PARAPET ELEVATION
LEFT REAR SHOWN, OTHERS SIMILAR



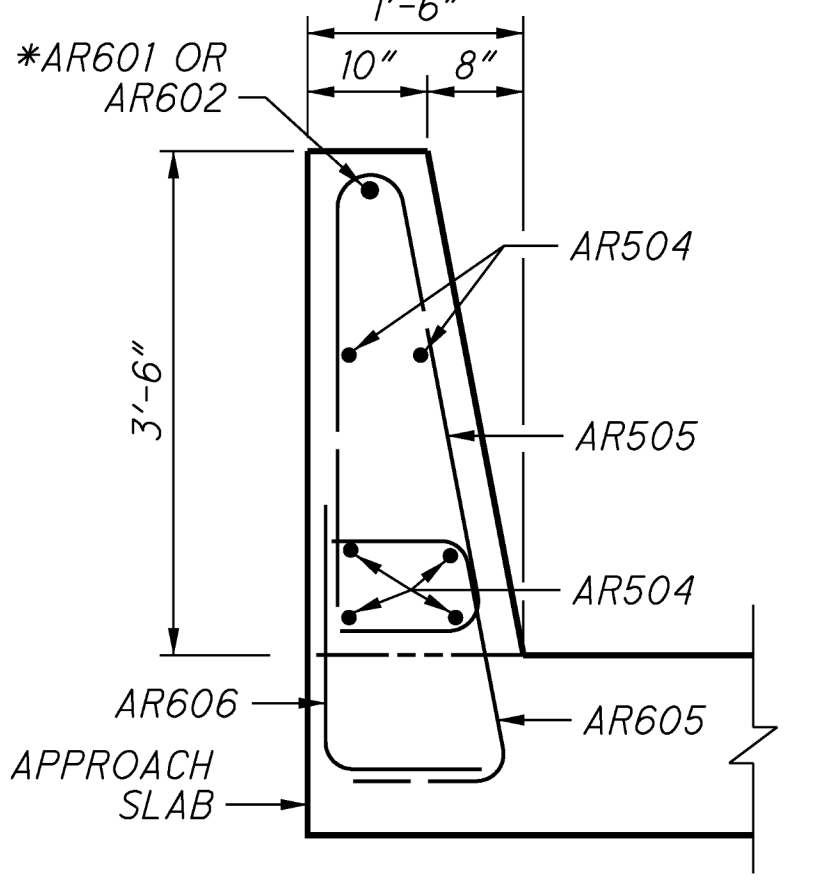
A SECTION
25 * FIELD BEND IF NECESSARY



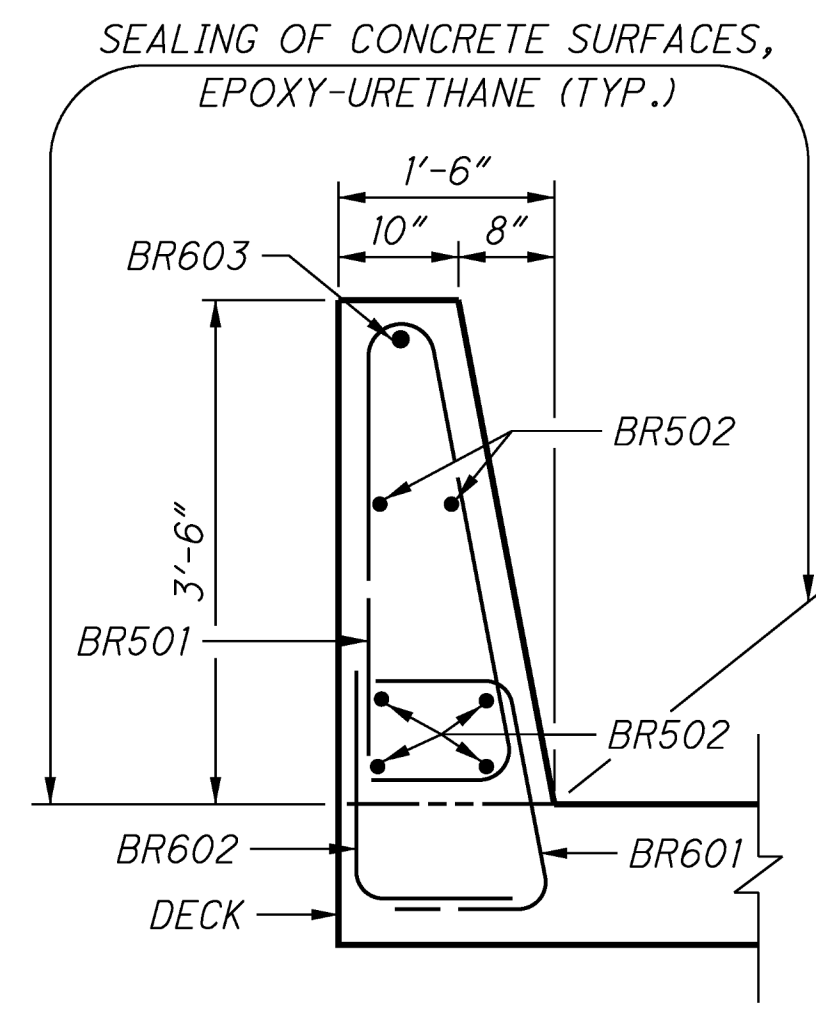
B SECTION
25



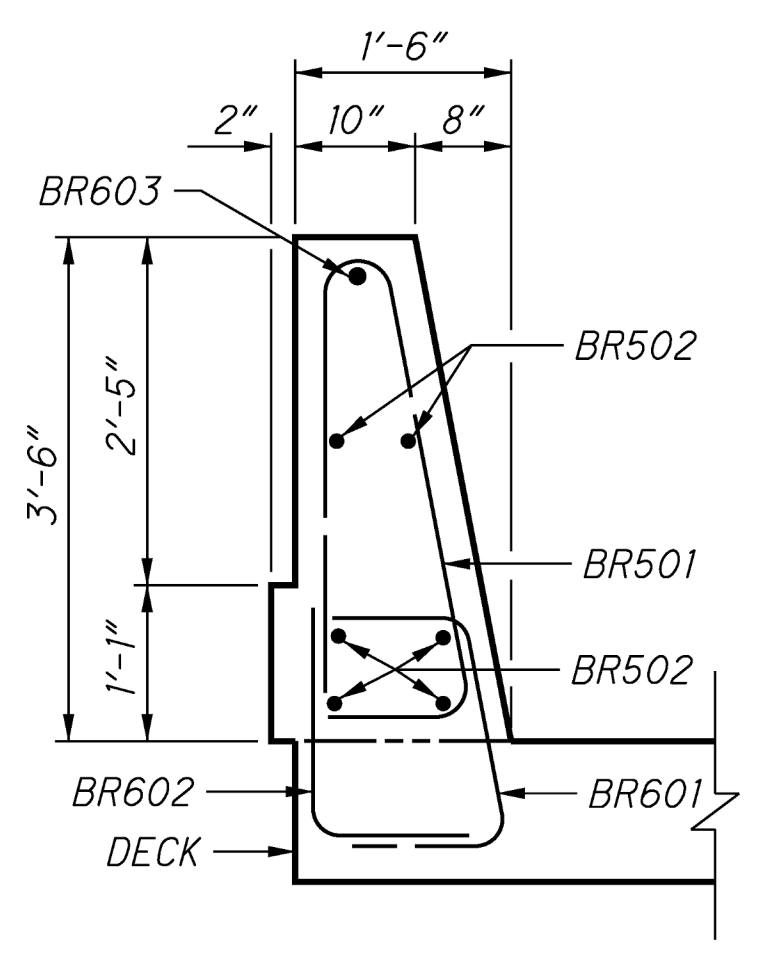
C SECTION
25



D SECTION
25



E SECTION
25



F SECTION
25

MINIMUM LAP LENGTH
NO. 5 BAR = 29"
NO. 6 BAR = 49"

NOTES:

- ALL PARAPETS AND REINFORCING STEEL IN PLAN AND ELEVATION VIEW ARE DIMENSIONED ALONG THE TOE OF THE PARAPET.
- TO ACCOUNT FOR SKEW, EXTRA BARS HAVE BEEN ADDED ON EACH SIDE OF THE APPROACH PARAPET/BRIDGE PARAPET JOINT AND ARE TO BE PLACED ON SKEW. ON EACH CORNER OF THE BRIDGE, A SET OF AR505, AR604, AND AR605 BARS SHALL BE SET ON THE APPROACH SIDE OF THE JOINT AND A SET OF BR501, BR601, AND BR602 BARS SHALL BE SET ON THE BRIDGE SIDE OF THE JOINT.
- CONCRETE AND REINFORCING FOR THE APPROACH SLAB MOUNTED PARAPETS AND TRANSITIONS SHALL BE INCLUDED FOR PAYMENT WITH ITEM 526, REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN. FOR APPROACH SLAB REINFORCING STEEL, SEE SCD AS-1-81.
- FOR BRIDGE TERMINAL ASSEMBLY, SEE SHEET 23 OF 52 AND SCD MGS-3.1.
- CONCRETE, CONTROL JOINTS, SAWCUT AND CAULKING MATERIAL FOR THE BRIDGE DECK MOUNTED PARAPETS SHALL BE INCLUDED FOR PAYMENT WITH ITEM 511, CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET).
- SEE SCD SBR-1-99 FOR ADDITIONAL PARAPET DETAILS.



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MARK	NUMBER				LENGTH	WEIGHT	TYPE	DIMENSION			
	PHASE 1		PHASE 2					TOTAL	A	B	C
	REAR	FORWARD	REAR	FORWARD							
SUBSTRUCTURE - DIAPHRAGMS & BREAST WALLS											
A501	32	32	36	36	136	6' - 9 1/2"	963	2	1' - 8"	3' - 8 1/2"	1' - 8"
A502	16	16	18	18	68	7' - 8"	544	2	2' - 7"	2' - 9"	2' - 7"
A503	2					4' - 6"			1' - 10 1/2"		
	SERIES OF				20	TO	106	1	TO	2' - 9"	1 1/2"
	10					5' - 7 1/2"			3' - 0"		
A504	12	14			57	5' - 4 1/2"	320	1	2' - 9"	2' - 9"	
A505	2	2			4	5' - 6"	23	1	2' - 9"	2' - 11 1/2"	
A506				2	2	7' - 2 1/2"	15	1	4' - 7"	2' - 9"	
A507			2			5' - 5"			2' - 9 1/2"		
			SERIES OF		30	TO	197	1	TO	2' - 9"	1 1/2"
			15			7' - 2"			4' - 6 1/2"		
A508			2		2	5' - 6 1/2"	12	1	2' - 11"	2' - 9"	
A509			2		2	5' - 8 1/2"	12	1	2' - 11"	2' - 11"	
A510		2				4' - 4 1/2"			1' - 9"		
		SERIES OF			20	TO	103	1	TO	2' - 9"	1 1/2"
		10				5' - 6"			2' - 10 1/2"		
A511			2			5' - 11 1/2"			3' - 4"		
			SERIES OF		32	TO	220	1	TO	2' - 9"	1"
			16			7' - 2 1/2"			4' - 7"		
A512			2		2	6' - 1 1/2"	13	1	3' - 4"	2' - 11"	
A513	1				1	23' - 2"	24	STR.			
A514	1				1	24' - 4"	25	STR.			
A515			2		2	18' - 8"	39	STR.			
A516			1	2	3	25' - 9"	81	STR.			
A517			1		1	24' - 10"	26	STR.			
A518		1			1	25' - 2"	26	STR.			
A519		1			1	23' - 9"	25	STR.			
A520				2	2	27' - 1"	56	STR.			
A578	6	6			12	5' - 1"	64	2	1' - 10"	1' - 8"	1' - 10"
A579	3	3			6	4' - 6"	28	2	1' - 10"	1' - 1"	1' - 10"
A580	3	3			6	5' - 0"	31	2	1' - 10"	1' - 7"	1' - 10"
A801*	3	3			6	19' - 7"	314	19	14' - 1"	5' - 6"	8"
A802	3				3	8' - 8"	69	STR.			
A803*			14		14	26' - 2"	978	STR.			
A804*	11				11	22' - 11"	673	STR.			
A805		3			3	9' - 4"	75	STR.			
A806*		11			11	23' - 5"	688	STR.			
A807*				14	14	26' - 10"	1003	STR.			
A808	4	6			10	15' - 6"	414	STR.			
A809*	4				4	15' - 0"	160	STR.			
A810			4		4	9' - 4"	100	STR.			
A811*			4	6	10	21' - 3"	567	STR.			
A812*		6			6	15' - 6"	248	STR.			
A813				4	4	10' - 0"	107	STR.			
D801	15	16	17	18	66	5' - 0"	881	18	2' - 10"	1' - 0"	1' - 0"
SUB-TOTAL FOR SUBSTRUCTURE - DIAPHRAGMS & BREAST WALLS						9,230					

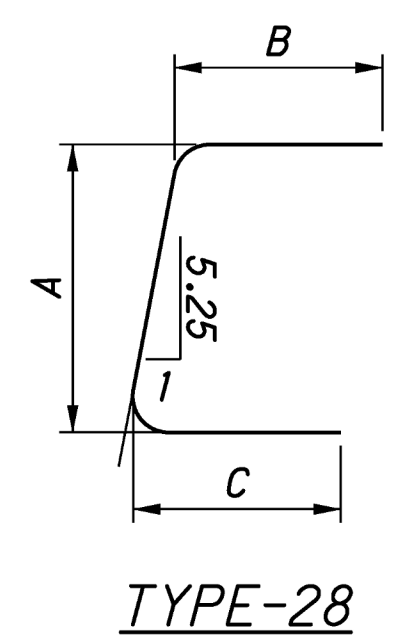
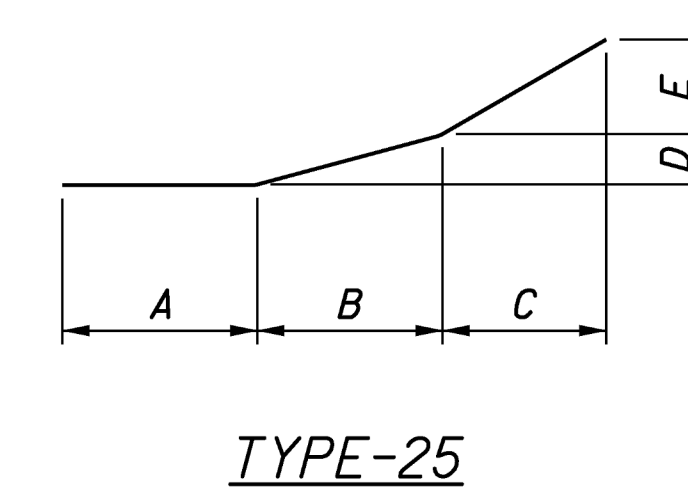
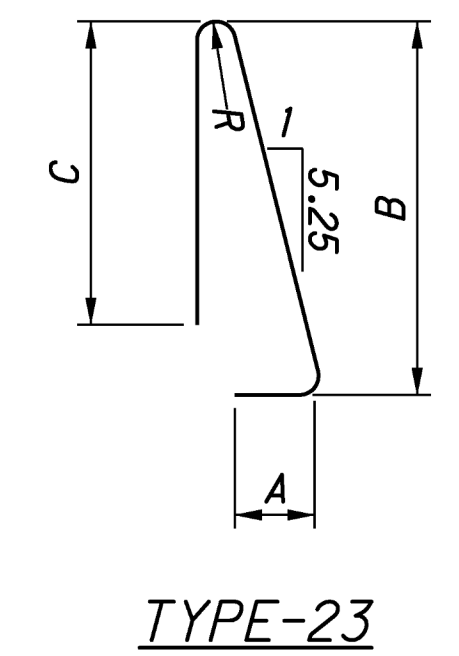
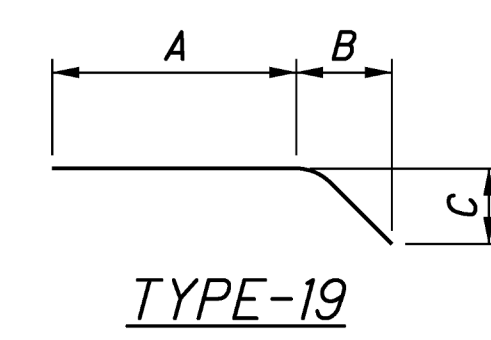
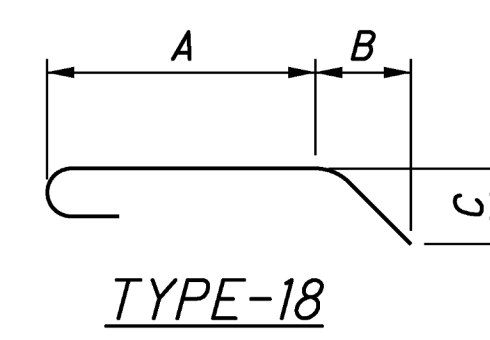
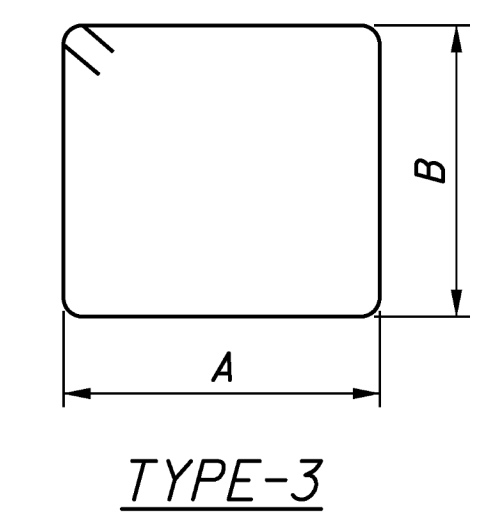
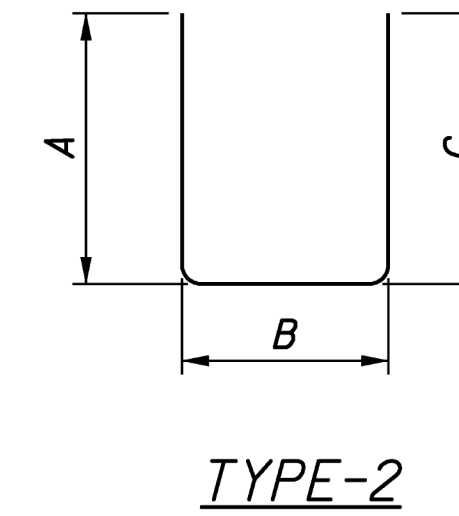
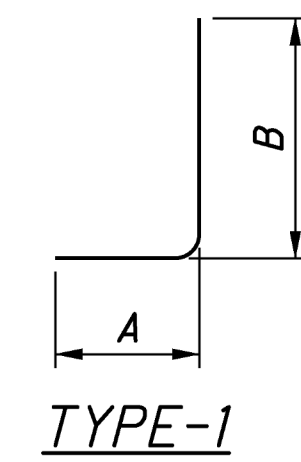
REINFORCING STEEL NOTES:

1. THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST DIGIT WHERE THREE DIGITS ARE USED, AND THE FIRST TWO DIGITS WHERE FOUR ARE USED, INDICATES THE BAR SIZE NUMBER. FOR EXAMPLE S501 IS A NO. 5 BAR. BAR DIMENSIONS SHOWN ARE OUT TO OUT UNLESS OTHERWISE NOTED. R INDICATES INSIDE RADIUS, UNLESS OTHERWISE NOTED.
2. ALL REINFORCING STEEL SHALL BE EPOXY COATED.
3. "STR." IN THE TYPE COLUMN INDICATES STRAIGHT BARS.
4. REFER TO CMS SECTION 509.05 FOR STANDARD BEND DIMENSIONS.
5. * - REQUIRES MECHANICAL CONNECTORS.

MECHANICAL CONNECTORS:

AN APPROVED TYPE OF MECHANICAL CONNECTOR FOR REINFORCING SHALL BE PROVIDED. INSTALLATION OF THE CONNECTORS SHALL CONFORM WITH RECOMMENDED MANUFACTURER'S PROCEDURES. IF A DOWEL BAR SPLICE IS FURNISHED, THE MINIMUM DOWEL BAR LENGTH TO BE INCLUDED WITH THE CONNECTOR SHALL BE GIVEN BY THE DIMENSION "L" SHOWN BELOW:

- # 4 REINFORCING BAR L = 2'-3"
- # 5 REINFORCING BAR L = 2'-11"
- # 6 REINFORCING BAR L = 3'-5"
- # 8 REINFORCING BAR L = 5'-9"



DESIGN AGENCY O.D.O.T. - DISTRICT 11 PLANNING & ENGINEERING
DATE STRUCTURE FILE NUMBER 340200
REVIEWED DRAWN CCN REVISED
DESIGNED CCN CHECKED RPT
REINFORCING STEEL LIST BRIDGE NO. HAS-22-2126 OVER WHEELING & LAKE ERIE RAILROAD
HAS-22-21.26 PID No. 88904
26/29

ITEM 509, EPOXY COATED REINFORCING STEEL
9,230 LB. (CARRIED TO GENERAL SUMMARY)

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MARK	NUMBER				LENGTH	WEIGHT	TYPE	DIMENSION				
	PHASE 1		PHASE 2					TOTAL	A	B	C	D
	REAR	FORWARD	REAR	FORWARD								
SUBSTRUCTURE - FOOTINGS & WING WALLS (1 of 2)												
A521	6		5		11	16' - 8"	191	3	2' - 7"	5' - 5"		
A522	1		1		2	17' - 2"	36	3	2' - 7"	5' - 8"		
A523	7				7	20' - 11"	153	2	9' - 6"	2' - 2"	9' - 6"	
A524	1				1	14' - 9"	15	2	6' - 4"	2' - 4"	6' - 4"	
A525			2		2	9' - 11"	21		4' - 0"	2' - 2"	4' - 0"	
A526	6	5	3	2	16	11' - 9"	196	2	4' - 11"	2' - 2"	4' - 11"	
A527	2	1	2	1	6	11' - 11"	75	2	4' - 11"	2' - 4"	4' - 11"	
A528	1				1	7' - 10"	8	STR.				
A529	1				1	9' - 9"	10	STR.				
A530	3	1			4	10' - 0"	42	STR.				
A531	3				3	9' - 3"	29	STR.				
A532	2				2	10' - 4"	22	STR.				
A533	2			2	4	9' - 7"	40	STR.				
A534	1	2			3	8' - 6"	27	STR.				
A535	1				1	7' - 8"	8	STR.				
A536	1				1	6' - 8"	7	STR.				
A537	1				1	5' - 11"	6	STR.				
A538	1				1	8' - 11"	9	19	3' - 5"	5' - 0"	2' - 3"	
A539	1				1	8' - 2"	9	19	2' - 8"	5' - 0"	2' - 3"	
A540			6		6	16' - 1"	101	2	7' - 1"	2' - 2"	7' - 1"	
A541			1		1	9' - 11"	10	2	4' - 0"	2' - 2"	4' - 0"	
A542			1		1	6' - 6"	7	STR.				
A544			2		2	8' - 3"	17	STR.				
A545			1		1	9' - 1"	9	STR.				
A546				2	2	10' - 7"	22	STR.				
A547			1	1	2	6' - 9"	14	STR.				
A548		1	1		2	7' - 6"	16	STR.				
A549			1		1	5' - 0"	5	STR.				
A550			1	1	2	5' - 9"	12	STR.				
A551			1		1	7' - 2"	7	19	1' - 8"	5' - 0"	2' - 4"	
A552			1		1	8' - 0"	8	19	2' - 6"	5' - 0"	2' - 4"	
A553		5		5	10	16' - 0"	167	3	2' - 7"	5' - 8"		
A554		1		1	2	17' - 0"	35	3	2' - 7"	6' - 2"		
A555		6			6	18' - 3"	114	2	8' - 2"	2' - 2"	8' - 2"	
A556				1	4	7' - 5"	39	2	2' - 9"		2' - 9"	
				SERIES OF	4	TO			TO	2' - 2"	TO	1' - 4"
				4		11' - 5"			4' - 9"		4' - 9"	
A558		1			1	7' - 8"	8	STR.				
A559		2			2	9' - 1"	19	STR.				
A560		2			2	8' - 2"	17	STR.				
A561		2	2		4	9' - 4"	39	STR.				
A562		1			1	6' - 7"	7	STR.				
A563		1			1	4' - 9"	5	STR.				
A564		1			1	5' - 8"	6	STR.				
A565		1			1	7' - 0"	7	19	1' - 6"	5' - 0"	2' - 4"	
A566		1			1	7' - 11"	8	19	2' - 5"	5' - 0"	2' - 4"	
SUB-TOTAL FOR SUBSTRUCTURE - FOOTINGS & WING WALLS (1 of 2)							1,603					

NOTES

1. FOR BENDING DIAGRAM AND ADDITIONAL NOTES, SEE SHEET 26/29.

DESIGNED CCN	CHECKED RPT	DRAWN CCN	REVISED	REVIEWED	DATE	STRUCTURE FILE NUMBER 340200	DESIGN AGENCY O.D.O.T. DISTRICT 11 PLANNING & ENGINEERING
REINFORCING STEEL LIST							
BRIDGE NO. HAS-22-2126							
OVER WHEELING & LAKE ERIE RAILROAD							
HAS-22-21.26							
PID No. 88904							
27/29							
50							
52							

ITEM 509, EPOXY COATED REINFORCING STEEL
1,603 LB. (CARRIED TO GENERAL SUMMARY)

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MARK	NUMBER				LENGTH	WEIGHT	TYPE	DIMENSION				
								A	B	C	D	INC.
SUBSTRUCTURE - FOOTINGS & WING WALLS (2 of 2)												
	PHASE 1		PHASE 2		TOTAL							
	REAR	FORWARD	REAR	FORWARD								
A567				7	7	13' - 5"	98	2	5' - 9"	2' - 2"	5' - 9"	
A568				2	2	10' - 8"	22	STR.				
A569				2	2	8' - 5"	18	STR.				
				1		8' - 10"						
A570				SERIES OF 5	5	TO 11' - 2"	52	STR.				
A571			2	1	3	8' - 8"	27	STR.				
A572				1	1	7' - 9"	8	STR.				
A573				1	1	9' - 1"	9	19	3' - 7"	5' - 0"	2' - 4"	
A574				1	1	8' - 2"	9	19	2' - 8"	5' - 0"	2' - 4"	
	1					7' - 10"						
A575	SERIES OF 4				4	TO 9' - 7"	36	STR.			7"	
			1			6' - 6"						
A576			SERIES OF 4		4	TO 8' - 3"	31	STR.			7"	
		1				7' - 8"						
A577		SERIES OF 5			5	TO 10' - 0"	46	STR.			7"	
A581		1		1	2	9' - 4"	19	2	3' - 6"	2' - 7"	3' - 6"	
	1					8' - 4"						
A814	SERIES OF 4				4	TO 10' - 1"	38	STR.			7"	
			1			7' - 0"						
A815			SERIES OF 4		4	TO 8' - 9"	33	STR.			7"	
		1				8' - 2"						
A816		SERIES OF 5			5	TO 10' - 6"	49	STR.			7"	
				1		8' - 10"						
A817				SERIES OF 5	5	TO 11' - 2"	52	STR.			6"	
SUB-TOTAL FOR SUBSTRUCTURE - FOOTINGS & WING WALLS (2 of 2)						547						

MARK	NUMBER				LENGTH	WEIGHT	TYPE	DIMENSION				
								A	B	C	D	RADIUS
BRIDGE RAILING												
	PHASE 1		PHASE 2		TOTAL							
	LEFT	RIGHT	LEFT	RIGHT								
BR501		151	150		301	7' - 4"	2302	23	1' - 0"	3' - 2"	3' - 0"	
BR502		36	36		72	27' - 0"	2028	STR.				
BR601		151	150		301	3' - 2"	1432	28	1' - 8"	11"	1' - 1"	
BR602		151	150		301	2' - 8"	1206	1	1' - 1"	1' - 9"		
BR603		6	6		12	28' - 3"	509	STR.				
SUB-TOTAL FOR BRIDGE RAILING						7,477						

NOTES

1. FOR BENDING DIAGRAMS AND ADDITIONAL NOTES, SEE SHEET 26/29.

REINFORCING STEEL LIST
 BRIDGE NO. HAS-22-2126
 OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26
 PID No. 88904

28/29

51
52

ITEM 509, EPOXY COATED REINFORCING STEEL
 547 LB. + 7,477 LB. = 8,024 LB. (CARRIED TO GENERAL SUMMARY)

DESIGN AGENCY
 O.D.O.T. DISTRICT 11
 PLANNING & ENGINEERING

DATE
 REVIEWED
 STRUCTURE FILE NUMBER
 340200

DRAWN
 CCN
 CHECKED
 RPT

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MARK	NUMBER			LENGTH	WEIGHT	TYPE	DIMENSION				
							A	B	C	D	INC.
SUPERSTRUCTURE											
	PHASE 1		PHASE 2	TOTAL							
S401	120		135	255	31' - 3"	5323	STR.				
S601	46		52	98	20' - 0"	2944	STR.				
S602	23		26	49	33' - 4"	2453	STR.				
S603	23		26	49	30' - 6"	2245	STR.				
S501	150		185	335	31' - 7"	11035	STR.				
S502	558			558	21' - 3 1/2"	12392	STR.				
S503			558	558	24' - 3 1/2"	14138	STR.				
	2				10' - 0"						
S504	SERIES OF 10			20	TO	326	STR.				1' - 3"
	2				0' - 10"						
S505	SERIES OF 19			38	TO	419	STR.				1' - 1"
	2				1' - 6"						
S506	SERIES OF 19			38	TO	505	STR.				1' - 3"
	2				8' - 1"						
S507	SERIES OF 16			32	TO	541	STR.				1' - 1"
S508	8			8	8' - 8"	72	STR.				
S509			8	8	7' - 4"	61	STR.				
SUB-TOTAL FOR SUPERSTRUCTURE						52,454					

NOTES

1. FOR BENDING DIAGRAM AND ADDITIONAL NOTES, SEE SHEET 26/29.

MARK	NUMBER				LENGTH	WEIGHT	TYPE	DIMENSION					
								A	B	C	D	E	RADIUS
APPROACH SLAB RAILING (FOR INFORMATION ONLY)													
	PHASE 1		PHASE 2		TOTAL								
	REAR RIGHT	FWD. RIGHT	REAR LEFT	FWD. LEFT									
AR501	5	5	5	5	20	5' - 6"	115	STR.					
AR502	3	3	3	3	12	5' - 7"	70	25	1' - 8"	2' - 5"	1' - 5"	1 1/2"	
AR503	8	8	8	8	32	10' - 0"	334	STR.					
AR504	6	6	6	6	24	13' - 8"	342	STR.					
AR505	14	15	15	14	58	7' - 4"	444	23	1' - 0"	3' - 2"	3' - 0"	2 3/4"	
AR601		1	1		2	15' - 3"	46	19	11' - 2"	4' - 1"	4"		
AR602	1			1	2	14' - 4"	43	19	10' - 3"	4' - 1"	4"		
AR603	8	8	8	8	32	4' - 2"	200	1	1' - 0"	3' - 4"			
	2	2	2	2		4' - 2"				3' - 4"			
AR604	S.O.	S.O.	S.O.	S.O.	88	TO	606	1	1' - 0"	TO		1"	
	11	11	11	11		5' - 0"				4' - 2"			
AR605	14	15	15	14	58	3' - 2"	276	28	1' - 8"	11"	1' - 1"		
AR606	14	15	15	14	58	2' - 9"	240	1	1' - 1"	1' - 10"			
SUB-TOTAL FOR APPROACH SLAB RAILING (FOR INFORMATION ONLY)							2,716						

REINFORCING STEEL LIST
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS - 22 - 21.26
PID No. 88904

29/29

52
52

ITEM 509, EPOXY COATED REINFORCING STEEL
52,454 LB. (CARRIED TO GENERAL SUMMARY)

DESIGN AGENCY
O.D.O.T. DISTRICT 11
PLANNING & ENGINEERING

REVIEWED
DATE
STRUCTURE FILE NUMBER
3401200

DRAWN
CCN
REVISIONS
CHECKED
RPT

PROJECT DESCRIPTION

THE PROJECT INVOLVES THE REHABILITATION OF A 3-SPAN BRIDGE THAT CARRIES US ROUTE 22 OVER WHEELING AND LAKE ERIE RAILROAD IN HARRISON COUNTY, OHIO. THE REHABILITATED STRUCTURE WILL BE A SEMI-INTEGRAL, 3-SPAN, CONTINUOUS COMPOSITE STEEL BEAM BRIDGE.

HISTORIC RECORDS

HISTORIC BORING RECORDS FOR THE EXISTING STRUCTURE WERE REQUESTED FROM THE ODOT OFFICE OF GEOTECHNICAL ENGINEERING. HOWEVER, NO HISTORIC BORING INFORMATION WAS LOCATED WITHIN THE LIMITS OF THE PROJECT.

GEOLOGY

ACCORDING TO THE OHIO DEPARTMENT OF NATURAL RESOURCES, PHYSIOGRAPHIC REGIONS OF OHIO, THE SITE LIES ON THE LITTLE SWITZERLAND PLATEAU, ALONG THE FLUSHING DIVIDE, IN THE UNGLACIATED PORTION OF EASTERN OHIO. ACCORDING TO BEDROCK GEOLOGIC MAP OF OHIO, BEDROCK BELOW THE SITE CONSISTS OF PENNSYLVANIAN AGE COAL, SANDSTONE, SHALE, AND LIMESTONE OF THE MONONGAHELA OR CONEMAUGH FORMATIONS.

RECONNAISSANCE

ON MAY 22, 2013, A RECONNAISSANCE VISIT WAS MADE. THE EXISTING PAVEMENT EXHIBITED SOME TRANSVERSE CRACKS IN AREAS BOTH EAST AND WEST OF THE BRIDGE. ALLIGATOR CRACKING WAS NOTED IN ISOLATED LOCATIONS EAST AND WEST OF THE BRIDGE. SOME SPALLING WAS NOTED ON THE BRIDGE DECK.

NO MAJOR SIGNS OF SLOPE INSTABILITY OR EROSION WERE NOTED IN THE EMBANKMENT WEST OF THE BRIDGE. SOME MINOR SLOUGHING WAS NOTED IN THE CUT SLOPE EAST OF THE BRIDGE AND SOUTH OF THE ROADWAY.

SUBSURFACE EXPLORATION

TWO (2) TEST BORINGS WERE PERFORMED FOR THIS PROJECT.

THE BORINGS WERE PERFORMED WITH A TRUCK-MOUNTED DRILL RIG UTILIZING HOLLOW STEM AUGERS (HSA) ON JUNE 18 AND 19, 2013. STANDARD PENETRATION TESTS WERE CONDUCTED USING A 140-POUND HAMMER FALLING 30 INCHES TO DRIVE A 2-INCH O.D. SPLIT BARREL SAMPLER. THE ENERGY TRANSFER RATIO ASSOCIATED WITH THE AUTOMATIC SPT HAMMER WAS 80.1 PERCENT. THE HAMMER WAS CALIBRATED IN OCTOBER 2011. ROCK CORING WAS PERFORMED IN BOTH TEST BORINGS USING AN NQ SIZE CORE BARREL WITH A DIAMOND BIT.

EXPLORATION FINDINGS

IN GENERAL, THE BORINGS EXHIBITED GRAVEL AND/OR STONE FRAGMENTS (A-1-a), SILT AND CLAY (A-6a), SILTY CLAY (A-6b) OR CLAY (A-7-6), DOWNWARDS TO THE DRILLED DEPTHS OF ALL ROADWAY BORINGS, AND TO DEPTHS RANGING FROM 58.5 TO 64.0 FEET IN CULVERT/BRIDGE BORINGS

BELOW THE SOIL OVERBURDEN, THE BORINGS EXHIBITED SEVERELY TO HIGHLY WEATHERED SHALE OR STRONG SANDSTONE. THE BEDROCK WAS SAMPLED USING ROCK CORING TECHNIQUES.

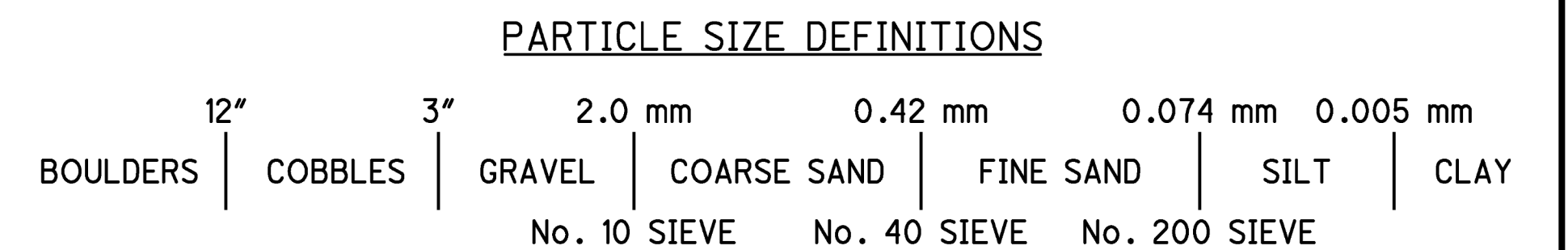
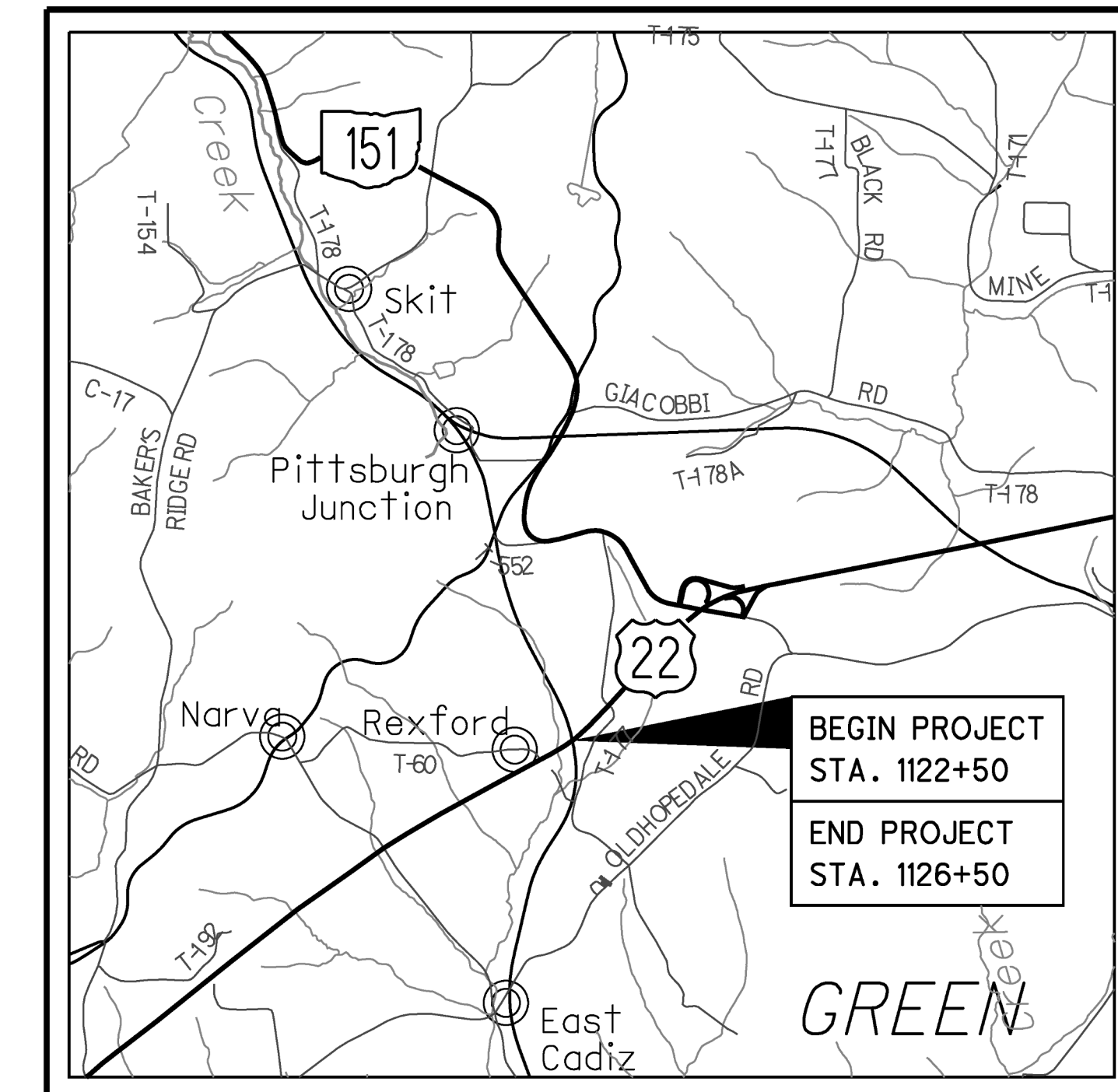
SPECIFICATIONS

THIS GEOTECHNICAL EXPLORATION WAS PERFORMED IN ACCORDANCE WITH THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, OFFICE OF GEOTECHNICAL ENGINEERING, SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS, DATED JANUARY 2013.

AVAILABLE INFORMATION

ALL AVAILABLE INFORMATION AND BEDROCK INFORMATION THAT CAN BE CONVINIENTLY SHOWN ON THE SOIL PROFILE SHEETS HAS BEEN REPORTED. ADDITIONAL SUBSURFACE EXPLORATIONS MAY HAVE BEEN MADE TO STUDY SOME SPECIAL ASPECT OF THE PROJECT. COPIES OF THIS DATA, IF ANY, MAY BE INSPECTED IN THE DISTRICT DEPUTY DIRECTOR'S OFFICE, THE OFFICE OF GEOTECHNICAL ENGINEERING AT 1600 WEST BROAD STREET OR THE OFFICE OF STRUCTURAL ENGINEERING AT 1980 WEST BROAD STREET.

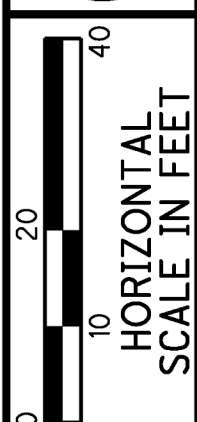
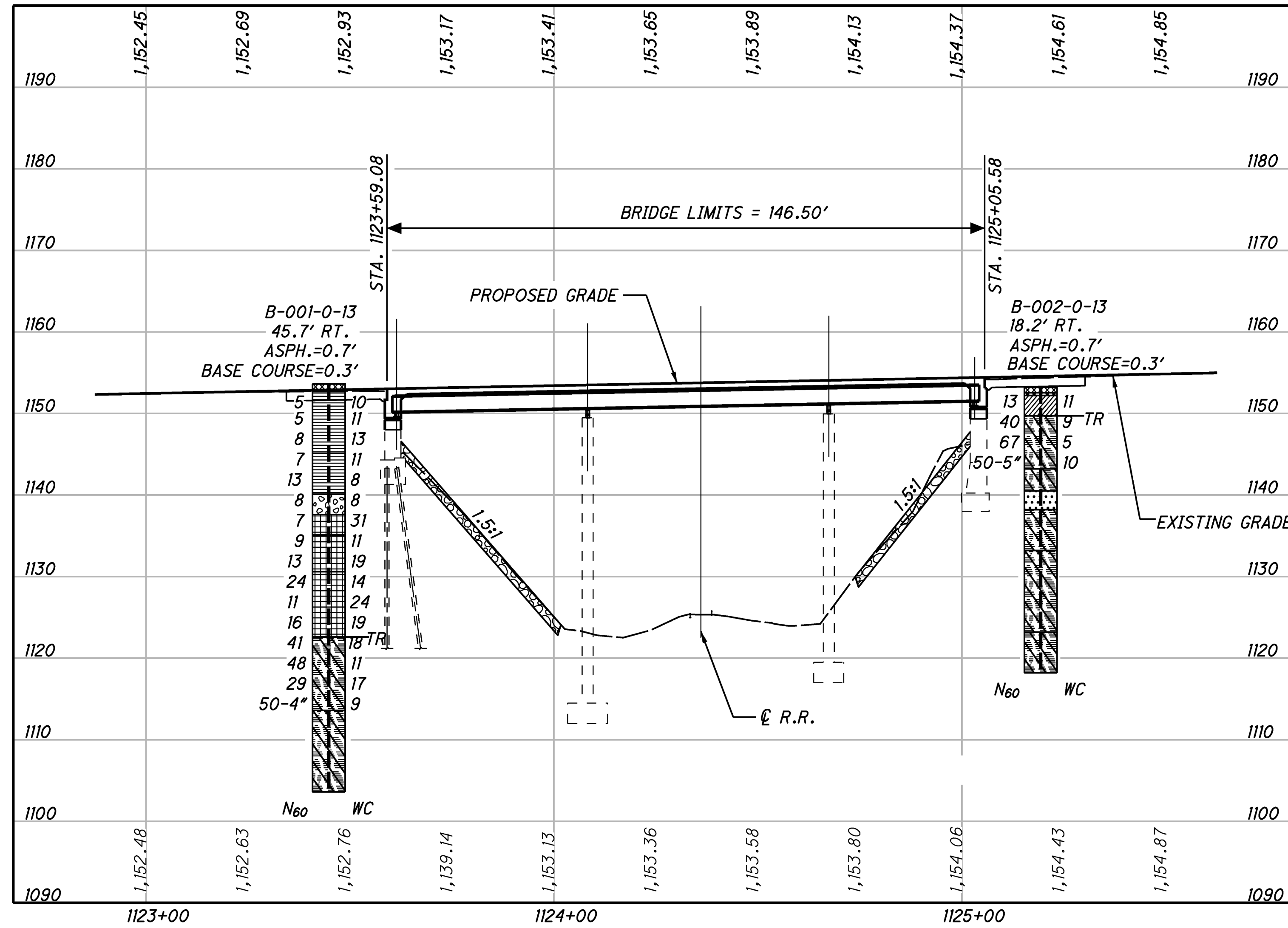
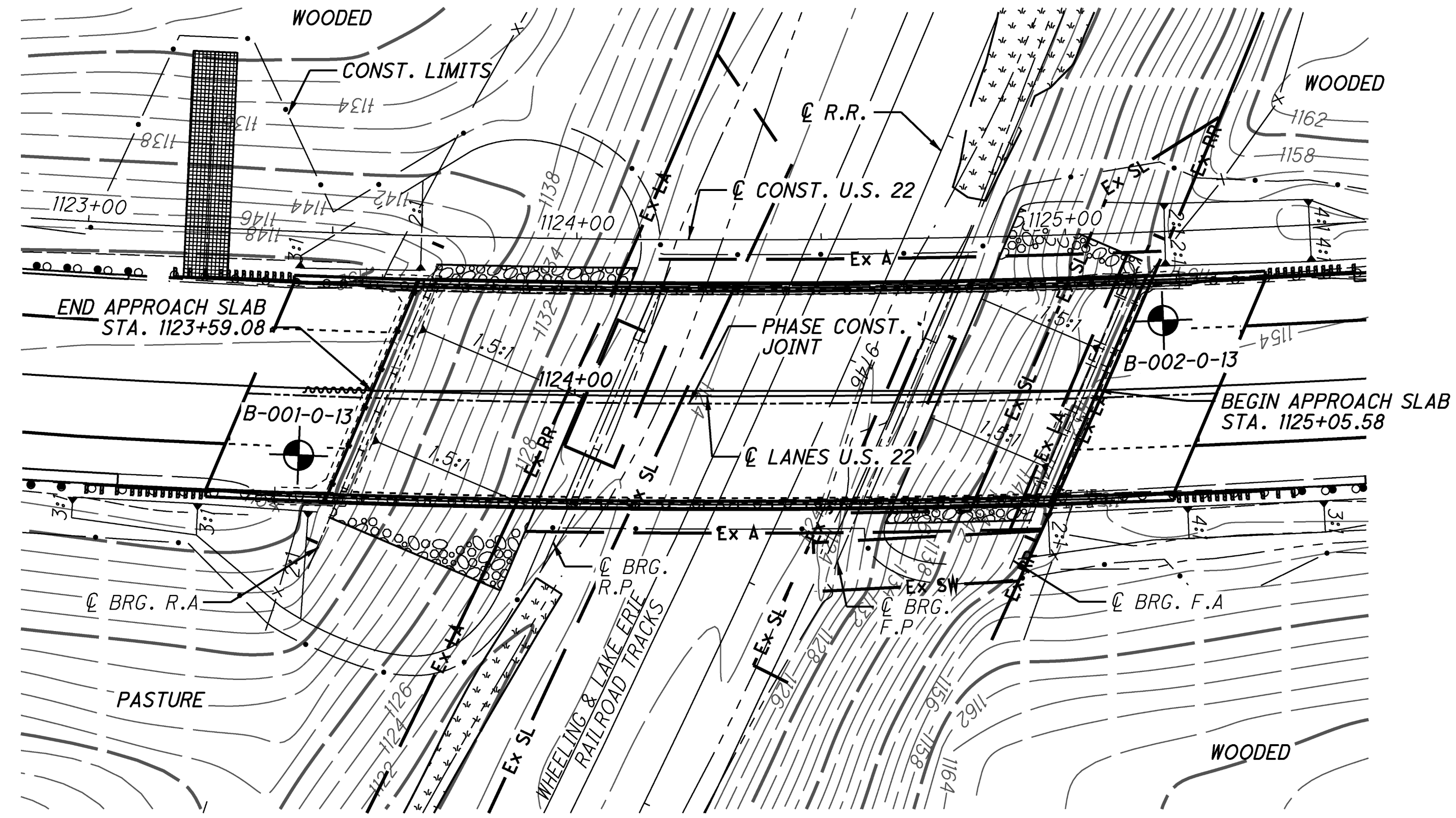
LEGEND		ODOT CLASS	CLASSIFIED MECH./VISUAL	
DESCRIPTION				
GRAVEL AND/OR STONE FRAGMENTS		A-1-a	0	1
SILT AND CLAY		A-6a	1	0
SILTY CLAY		A-6b	2	3
CLAY		A-7-6	3	3
		TOTAL	6	7
SANDSTONE		VISUAL		
WEATHERED SHALE		VISUAL		
PAVEMENT OR BASE = X = APPROXIMATE THICKNESS		VISUAL		
EXPLORATION LOCATION - PLAN VIEW				
DRIVE SAMPLE AND/OR ROCK CORE BORING PLOTTED TO VERTICAL SCALE ONLY. HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPHY.				
WC INDICATES WATER CONTENT IN PERCENT.				
INDICATES FREE WATER ELEVATION.				
N ₆₀ INDICATES STANDARD PENETRATION RESISTANCE NORMALIZED TO 60% DRILL ROD ENERGY RATIO.				
SS INDICATES A SPLIT-SPOON SAMPLE.				
TR INDICATES THE TOP OF ROCK.				



RECON. - JG 05/22/2013
 DRILLING - CTL ENGINEERING INC 06/18-06/19/13
 DRAWN - NKS 11/13/2013
 REVIEWED - JG 11/14/2013

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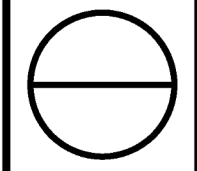
CTL ENGINEERING, INC.
 2860 FISHER ROAD
 COLUMBUS, OHIO 43204
 PHONE: (614) 276-8123 FAX: (614) 276-6377
 PID NO. 88904
STRUCTURE FOUNDATION EXPLORATION
 BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD
 HAS-22-21.26
 1/6



DRAWN N.K.S.
CHECKED NZ

STRUCTURE FOUNDATION EXPLORATION
BRIDGE NO. HAS-22-2126
OVER WHEELING & LAKE ERIE RAILROAD

HAS-22-21.26



PROJECT: HAS-22-21.26 TYPE: ROADWAY		DRILLING FIRM / OPERATOR: CTL / JP SAMPLING FIRM / LOGGER: CTL / JP		DRILL RIG: 0305R333-11 HAMMER: CME AUTOMATIC		STATION / OFFSET: 1123+44.8, 45.7		EXPLORATION ID B-001-0-13								
PID: 88904 BR ID: 6/19/13		DRILLING METHOD: 3.25" HSA / NQ		CALIBRATION DATE: 10/4/11		ALIGNMENT: US ROUTE 22		ELEVATION: 1153.6 (MSL), EOB: 50.0 ft.								
START: 6/19/13 END: 6/19/13		SAMPLING METHOD: SPT / NQ		ENERGY RATIO (%): 80.1		COORD: 240658.660 N, 2402383.642 E		PAGE 1 OF 1								
MATERIAL DESCRIPTION AND NOTES		ELEV.	SPT / RQD	REC N ₆₀ (%)	HP (tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	ODOT CLASS (G)	INST.
Asphalt (8")		1153.6	1													
Base course (3")		1152.9	2	5	67	16	9	15	32	28	36	20	16	10	A-6b (7)	
MEDIUM STIFF, BROWN, SILTY CLAY, SOME SAND, LITTLE GRAVEL, FILL, DAMP		1152.7	3													
@6.0': CONTAINS ROCK FRAGMENTS			4	5	56	-	-	-	-	-	-	-	-	11	A-6b (V)	
@8.5': VERY STIFF, SOME GRAVEL			5													
			6	8	22	-	-	-	-	-	-	-	-	13	A-6b (V)	
			7	3	3											
			8													
LOOSE, GRAY, GRAVEL AND/OR STONE FRAGMENTS, LITTLE CLAY, TRACE SAND, FILL, DAMP		1140.1	9	7	67	2.50	-	-	-	-	-	-	-	11	A-6b (V)	
			10													
			11													
VERY STIFF, BROWN, CLAY, AND SILT, LITTLE SAND, LITTLE GRAVEL, FILL, MOIST		1137.6	12	13	56	3.50	26	17	12	23	22	36	20	16	A-6b (4)	
			13													
			14	8	33	-	-	-	-	-	-	-	-	8	A-1-a (V)	
			15													
			16													
			17	7	78	3.00	11	8	10	36	35	43	23	20	A-7-6 (12)	
			18													
@18.5': STIFF, SOME GRAVEL WITH SHALE FRAGMENTS			19	9	100	-	-	-	-	-	-	-	-	11	A-7-6 (V)	
			20	4	3											
			21													
			22	13	44	2.00	-	-	-	-	-	-	-	19	A-7-6 (V)	
			23													
HARD, BROWN, CLAY, AND SILT, LITTLE SAND, TRACE GRAVEL WITH ORGANICS, DAMP		1130.6	24	24	100	4.50	2	5	10	46	37	45	22	23	A-7-6 (14)	
			25													
@26.0': MOIST			26	11	83	4.50	-	-	-	-	-	-	-	24	A-7-6 (V)	
			27	3	5											
			28													
@28.5': TRACE SAND, NO GRAVEL, DAMP			29	16	67	-	0	1	4	55	40	44	25	19	A-7-6 (12)	
			30	5	7											
SHALE, BROWN, SEVERELY WEATHERED, VERY WEAK.		1122.6	31													
			32	10	41	100	-	-	-	-	-	-	-	18	Rock (V)	
			33													
			34	15	48	100	-	-	-	-	-	-	-	11	Rock (V)	
			35													
			36	7	10	29	100	-	-	-	-	-	-	17	Rock (V)	
			37													
			38													
@38.5': GRAY.			39											9	Rock (V)	
			40													
SHALE, GRAY, SEVERELY TO HIGHLY WEATHERED, VERY WEAK.		1113.6	41													
			42	0	40	NQ-1									CORE	
			43													
			44													
			45													
			46													
			47													
			48	0	40	NQ-2									CORE	
			49													
			50													

NOTES: - GAVED AT 26'. GROUNDWATER SURFACE ELEVATION ASSUMED TO BE EQUAL TO 1153.6
ABANDONMENT METHODS, MATERIALS, QUANTITIES: SOIL CUTTINGS

PROJECT NO:	130500097
DATE:	7/8/2013

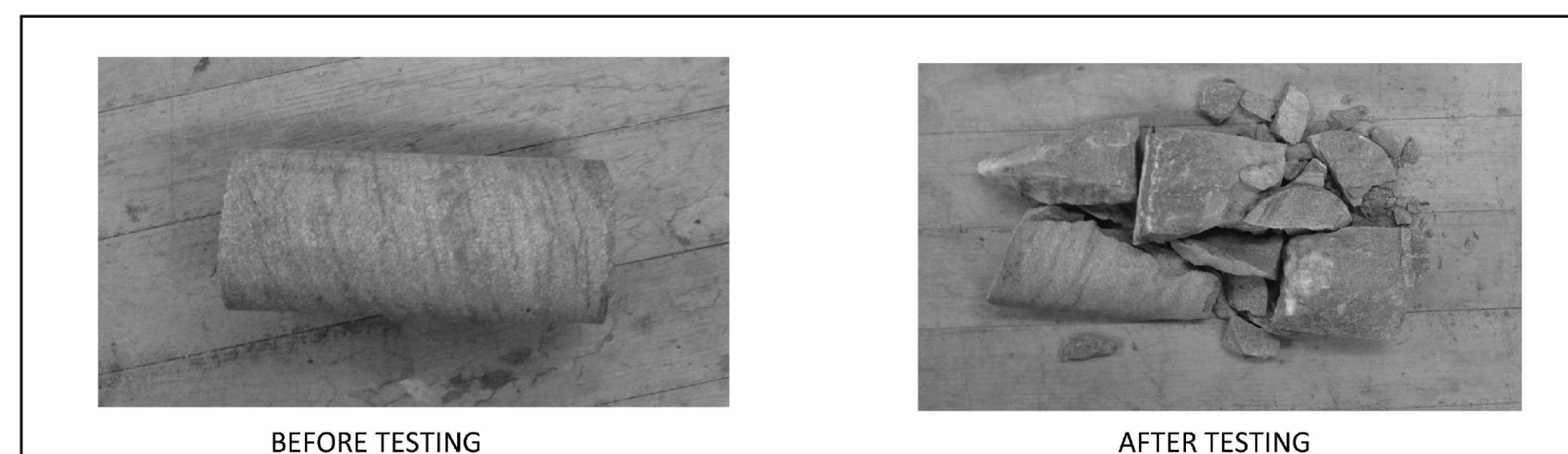
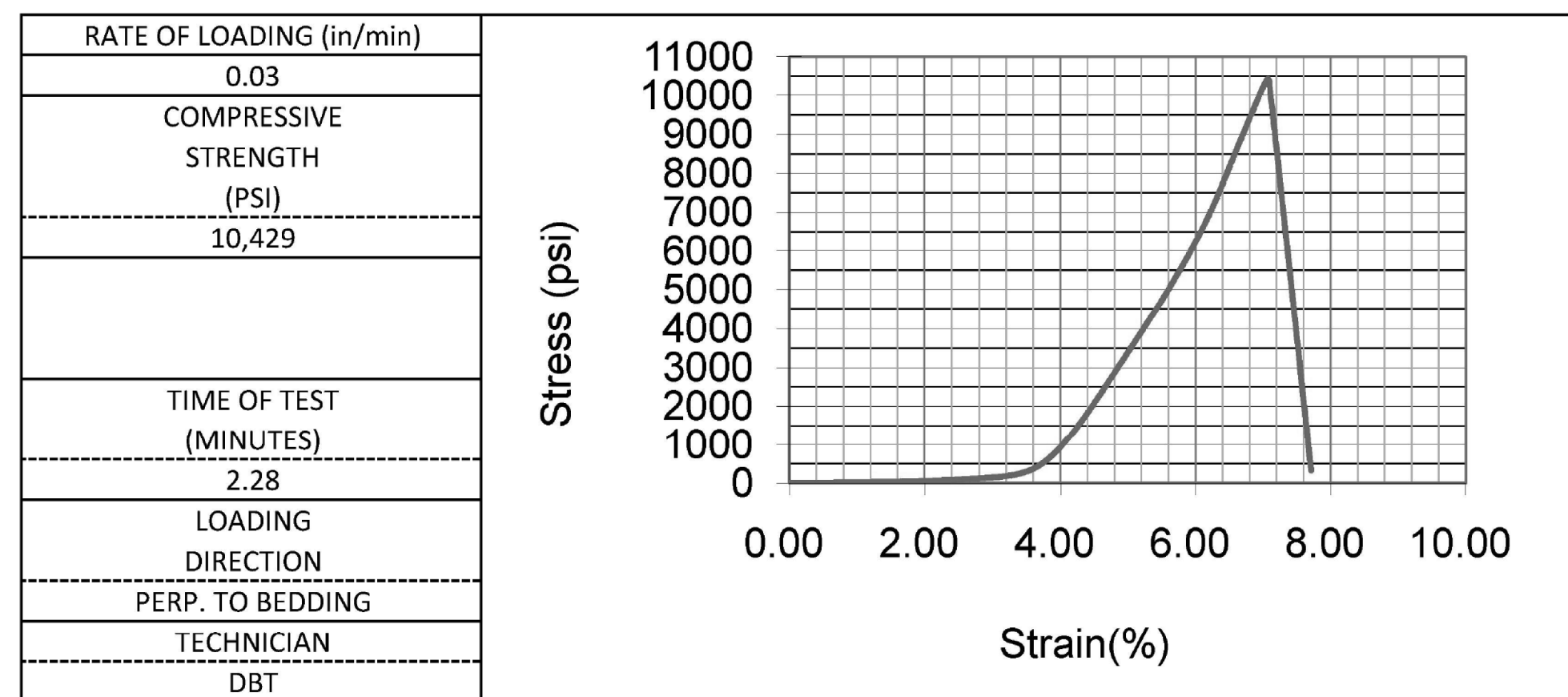
COMPRESSIVE STRENGTH AND ELASTIC MODULI OF ROCK - ASTM D 7012



BORING NUMBER	B-002-0-13	TOP DEPTH(FT)	14.0	BOTTOM DEPTH(FT)	14.4
SAMPLE NUMBER	RC-1	DISTRICT	11	PID	88904
COUNTY	Harrison	ROUTE	22	SECTION	21.26
STATION		OFFSET		OFFSET DIRECTION	

FORMATION	Conemaugh Group, (Upper Pennsylvanian)
DESCRIPTION	Sandstone, Gray, Slightly Weathered, Strong
MOISTURE CONDITION	As Received

MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)	LENGTH/DIAMETER	2.10
1	4.204	2.009	CORRECTION FACTOR	1
2	4.219	2.008	AREA(IN ²)	3.17
3	4.228	2.011	MASS (GRAMS)	575.7
AVERAGE	4.217	2.009	UNIT WEIGHT(LBS/FT ³)	164



PROJECT NO:	130500097
DATE:	7/8/2013

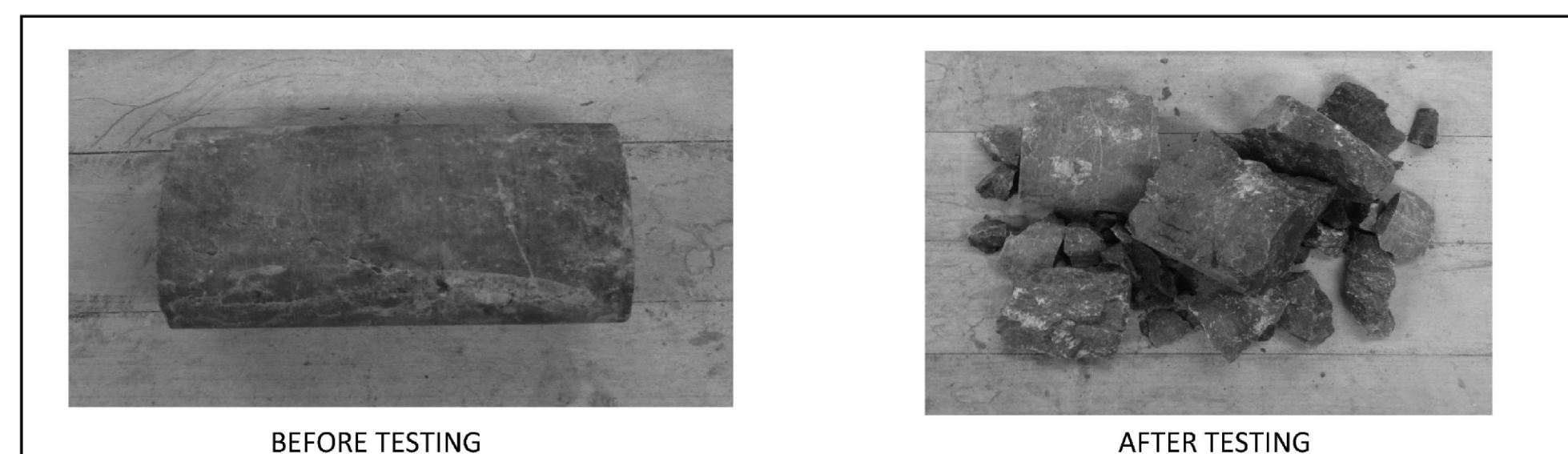
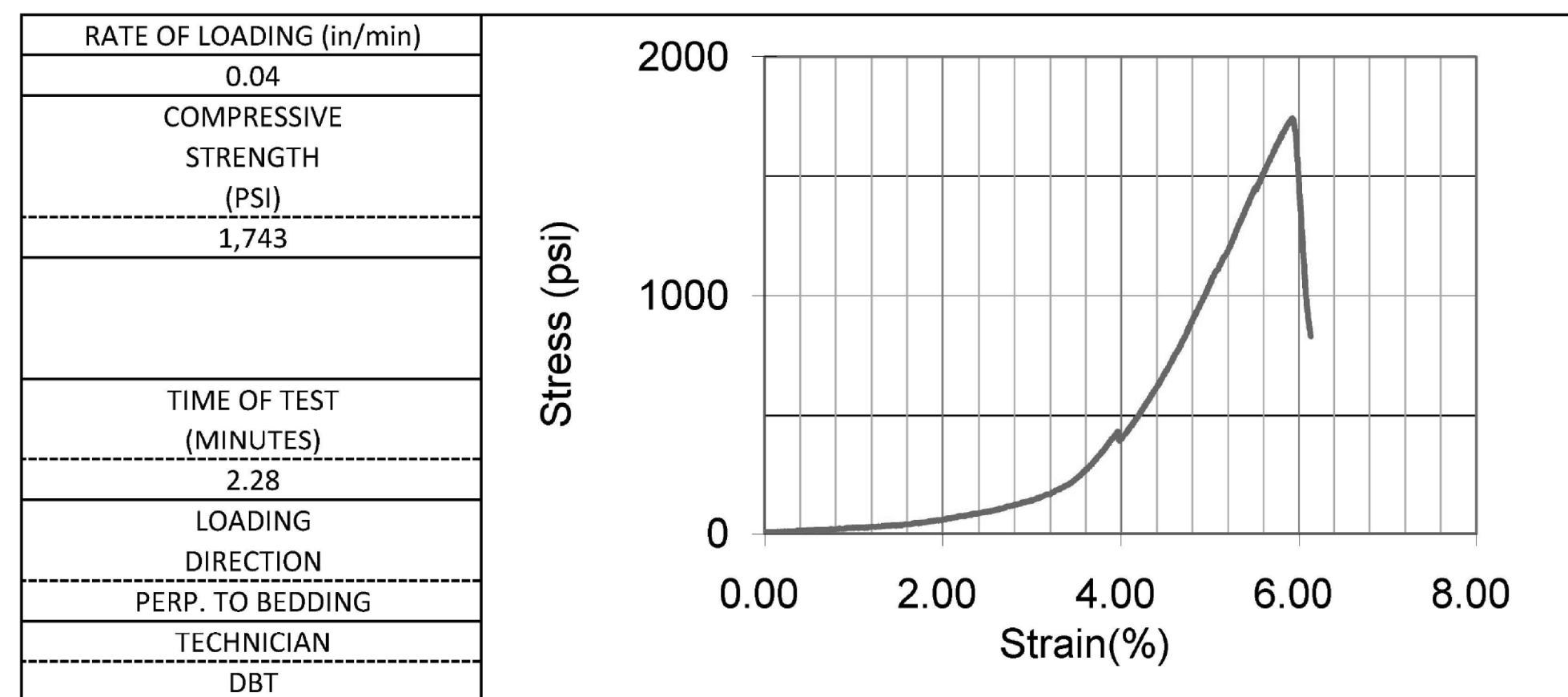
COMPRESSIVE STRENGTH AND ELASTIC MODULI OF ROCK - ASTM D 7012



BORING NUMBER	B-002-0-13	TOP DEPTH(FT)	25.0	BOTTOM DEPTH(FT)	25.4
SAMPLE NUMBER	RC-4	DISTRICT	11	PID	88904
COUNTY	Harrison	ROUTE	22	SECTION	21.26
STATION		OFFSET		OFFSET DIRECTION	

FORMATION	Conemaugh Group, (Upper Pennsylvanian)
DESCRIPTION	Shale, Brownish Gray, Highly Weathered, Slightly Strong
MOISTURE CONDITION	As Received

MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)	LENGTH/DIAMETER	1.98
1	3.902	1.988	CORRECTION FACTOR	1
2	3.969	1.989	AREA(IN ²)	3.11
3	3.97	1.99	MASS (GRAMS)	516
AVERAGE	3.947	1.989	UNIT WEIGHT(LBS/FT ³)	160.3

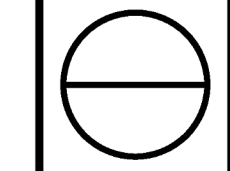


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STRUCTURE FOUNDATION EXPLORATION LABORATORY TEST DATA

HAS - 22 - 21.26

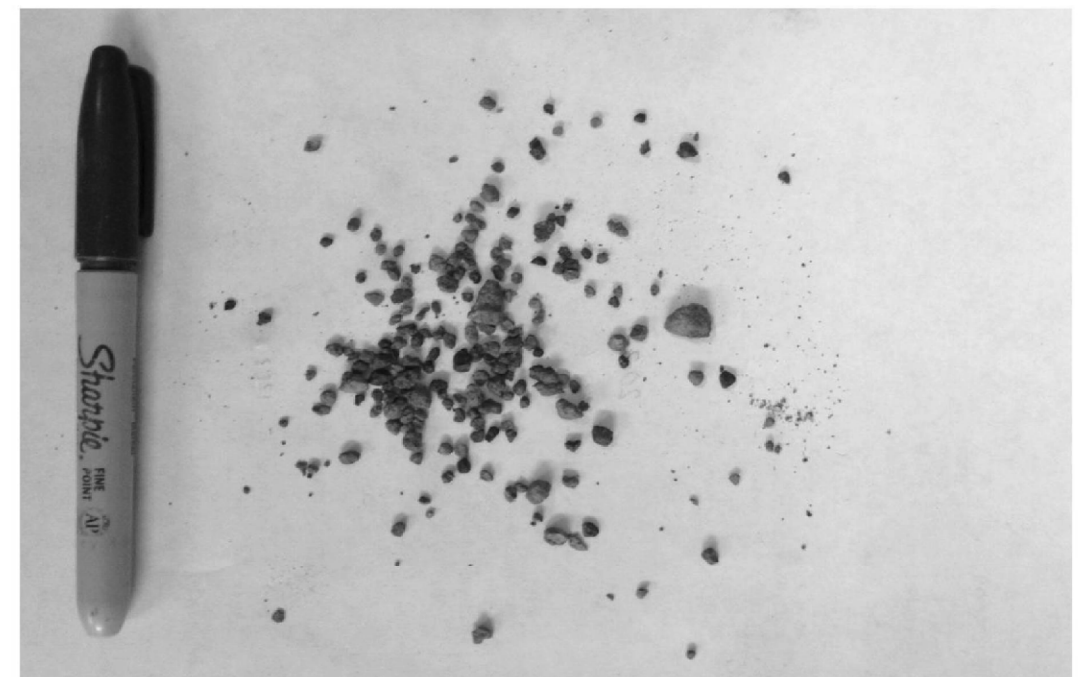


Slake Durability of Shales and Similar Weak Rocks ASTM D 4644-08	CTL ENGINEERING, INC. 2860 Fisher Road Columbus, Ohio 43204
<p>Sample ID: B-002-0-13 RC-1 Depth: 13.5'-14.0' Sample Description: Shale, Brown, Slightly Weathered, Slightly Strong</p> <p>Slake Durability Index (%): 86.0 (second cycle) Moisture Content (%): 4.0</p>	<p>Client: Ohio Department of Transportation District 11 Project: HAS-22-21.26 Location: Project No. 13050097COL Lab Code No. Date Tested: 7/2/13 Reviewed by: JG</p>



Type II—Retained specimen consist of large and small fragments.

Slake Durability of Shales and Similar Weak Rocks ASTM D 4644-08	CTL ENGINEERING, INC. 2860 Fisher Road Columbus, Ohio 43204
<p>Sample ID: B-002-0-13 RC-4 Depth: 29.5-30.0' Sample Description: Shale, Gray, Highly Weathered, Weak</p> <p>Slake Durability Index (%): 1.4 (second cycle) Moisture Content (%): 7.8</p>	<p>Client: Ohio Department of Transportation District 11 Project: HAS-22-21.26 Location: Project No. 13050097COL Lab Code No. Date Tested: 7/2/13 Reviewed by: JG</p>



Type III—Retained specimen is exclusively small fragments.