

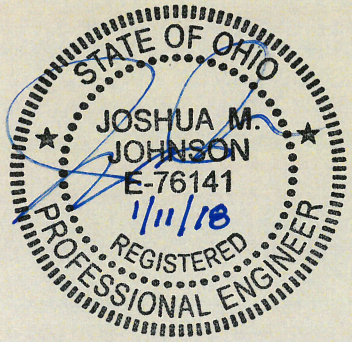




UNDERWATER BRIDGE INSPECTION REPORT

STRUCTURE NO. HAS-250-0934
SR 250 OVER TAPPAN LAKE RESERVOIR
HARRISON COUNTY, OH
DISTRICT 11

September 2017



Prepared for:

STATE OF OHIO DEPARTMENT OF TRANSPORTATION

Prepared by:

COLLINS ENGINEERS INC.

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EXECUTIVE SUMMARY

Project: ODOT District 11 Underwater Bridge Inspections - 2017

Purpose of Project: To perform a detailed visual and tactile underwater investigation of scour critical bridges for District 11 of the Ohio Department of Transportation.

Inspection Team: Team Leader – Kurt Lingo, E.I.T. – Collins Engineers, Inc.
Team Member – Matthew Rogers, E.I.T. – Collins Engineers, Inc.
Team Member – John Strub, E.I.T. – Collins Engineers, Inc.

Inspection Date(s):	August 2, 2017		
Water Visibility:	<1 ft	***Water Velocity:***	<1 ft/s
Water Temperature:	70 °F	***Weather:***	Sunny– 80 °F
Waterline Elevation:	898.4 ft	***Type of Boat:***	23 ft Carolina Skiff
Coordinates:	40.324190°N, 81.144059°W		
Access Location:	Shoreline 450 ft Northwest of HAS-250-0934		
Dive Mode:	Surface Supplied Air		
Waterline Reference:	9.8 ft below the top of slab at the midspan on the south fascia.		
Maximum Depth at SSU:	12.0 ft – Along Abutment 2		
Shoreline Conditions:	The east and west shorelines consisted of mild slope and heavy vegetation consisting of large trees.		

Summary of Findings:

- **Abutment/Wall 1:**
  - The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep
- **Abutment/Wall 2:**
  - Construction joint near south fascia is 1 in wide at channel bottom.
  - Construction joint near north fascia is ½ in wide at channel bottom.
  - The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep

Summary of Recommendations:

- Monitor scaling on Abutments 1 and 2.

Underwater Inspection Coding:

NBI Ratings:

Item	Description	Coding	Condition
60	Substructure	N/A	
61	Channel	7 – Good Condition	Banks and Channel are well protected.
62	Culvert	7 – Good Condition	Light scaling.
92B	UW Insp. Frequency	60 Months	
93B	Insp. Date	08 02 17	
113	Scour Critical Bridges	6 –Calculations not made	Stable (no indications of scour)

AASHTO National Bridge Element (NBE) Ratings:

Element #	Description	Units	Total	Condition State			
				1	2	3	4
241	Reinforced Concrete Culvert	FT	68	68	0	0	0

Note: Ratings were developed using the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.

1.0 INTRODUCTION

1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the SR 250 Bridge over Tappan Lake Reservoir in Harrison County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for the Ohio Department of Transportation (ODOT) on August 2, 2017. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

1.2 General Description of the Structure

Structure No. HAS-250-0934 spans 23.5 ft, carrying SR 250 over Tappan Lake Reservoir and is approximately 33.5 ft wide. The bridge superstructure is constructed of a cast-in-place concrete deck. The roadway orientation of the longitudinal axis of the bridge is west to east. The substructure units are labeled as Abutments 1 and 2. Previous inspection drawings were available at the time of the inspection and were used to determine water surface elevation and foundation elevations. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 and 2 in Exhibit 2 for overall views of the bridge.

1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.

A three-person team consisting of an engineer-diver and team leader (Kurt Lingo, E.I.T., P.E.) and two engineer divers (Matthew Rogers, E.I.T. and John Strub, E.I.T.) conducted the underwater inspection. The inspection was conducted using surface supplied air diving equipment. During the inspection, the inspectors worked from a boat and a note taker in the boat recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. Underwater photographs could not be obtained due to poor water conditions. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod, digital fathometer, and/or pneumofathometer. Soundings were collected at quarter points along the bridge centerline as well as 10 feet and 50 feet upstream and downstream of the bridge, parallel to the bridge fascias. Additional soundings were collected adjacent to Abutments 1 and 2 and at 10 feet intervals in-line with the abutments, upstream and downstream, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 7 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

2.0 EXISTING CONDITIONS

2.1 General Conditions

At the time of the inspection, the waterline of HAS-250-0934 was located approximately 9.8 ft below the top of slab at the midspan on the downstream (South) fascia, which corresponds to a waterline elevation of 898.40 ft. During the inspection, the waterway was flowing at approximately 0.5 ft per second. The culvert was well aligned with the reservoir. The east and west shorelines consisted of heavy vegetation (bushes/trees) with mild slopes. Refer to Photographs 3 through 6 in Exhibit 2 for views of the shorelines near the structure.

2.2 Substructure Conditions

2.2.1 Abutment/Wall 1

The channel bottom material around Abutment 1 consisted of soft silt. The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Abutment 1. Refer to Photograph 7 in Exhibit 2 for views of Abutment 1 and typical concrete condition at the waterline.

2.2.2 Abutment/Wall 2

The channel bottom material around Abutment 2 consisted of soft silt. . The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Abutment 2. Refer to Photographs 8 and 9 in Exhibit 2 for views of Abutment 2 and typical concrete condition at the waterline. A construction near south fascia is 1 in wide at channel bottom. A construction joint near north fascia is ½ in wide at channel bottom.

3.0 EVALUATION AND RECOMMENDATIONS

Overall, the inspected substructure units of Structure No. HAS-250-0934 were in good condition. A comparison of the soundings recorded during the previous inspection on September 21, 2012 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. Although no channel deficiencies were observed, the channel bottom should continue to be monitored during future underwater inspections to verify that localized scour or overall channel degradation is not occurring.

It is recommended that the submerged substructure units of Structure No. HAS-250-0543 be next inspected underwater at an interval not to exceed 60 months, no later than August 2, 2023.

Respectfully Submitted,
COLLINS ENGINEERS, INC.

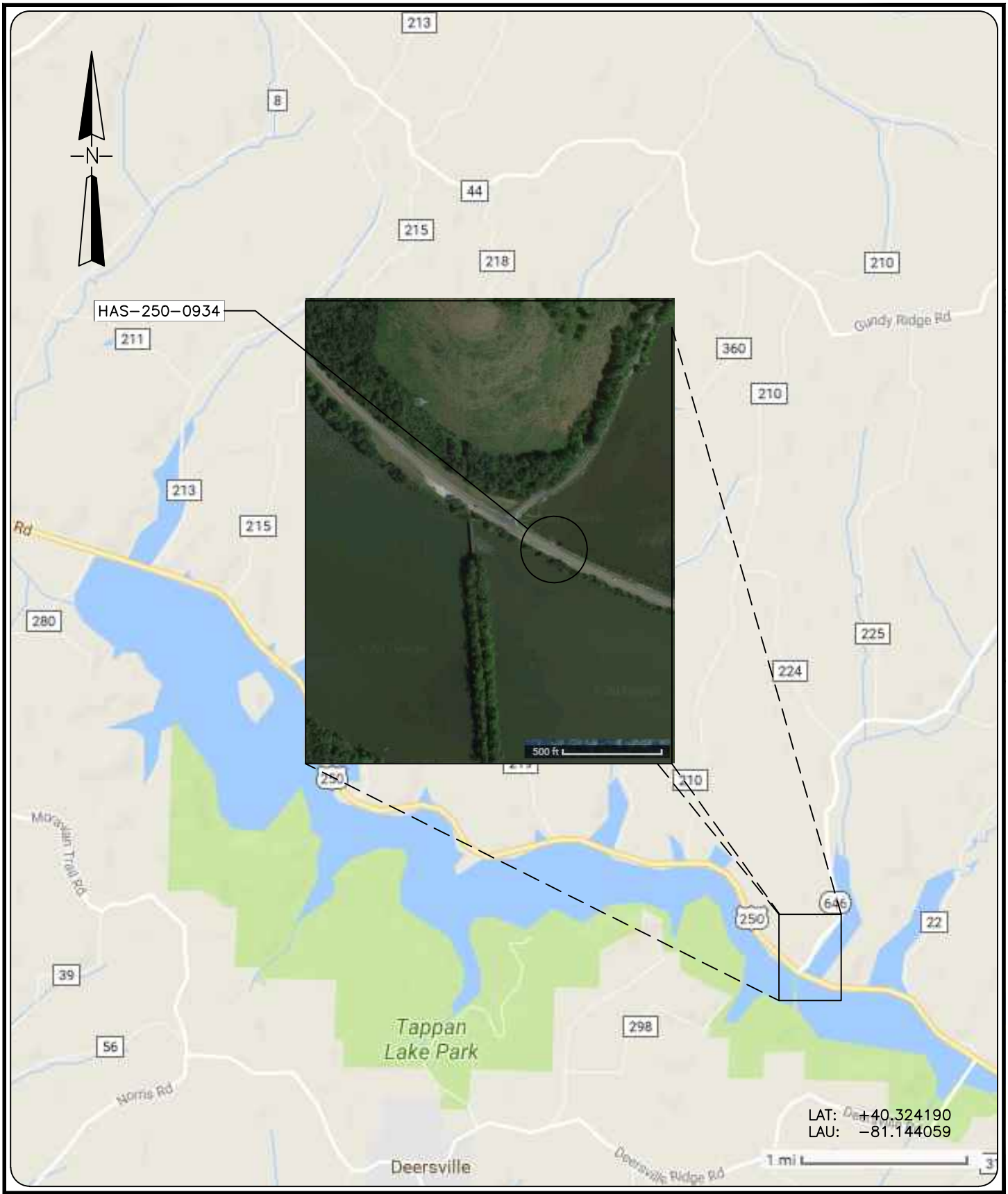
Joshua Johnson, P.E.

Project Manager/Team Leader

Originated by:

Matthew Rogers, E.I.T.

EXHIBIT 1 – FIGURES



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SR 250 OVER TAPPAN LAKE RESERVOIR
BRIDGE NO. HAS-250-0934

LOCATION MAP

HARRISON COUNTY, OHIO

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DRAWN BY: BBM	DATE: JUNE 2017
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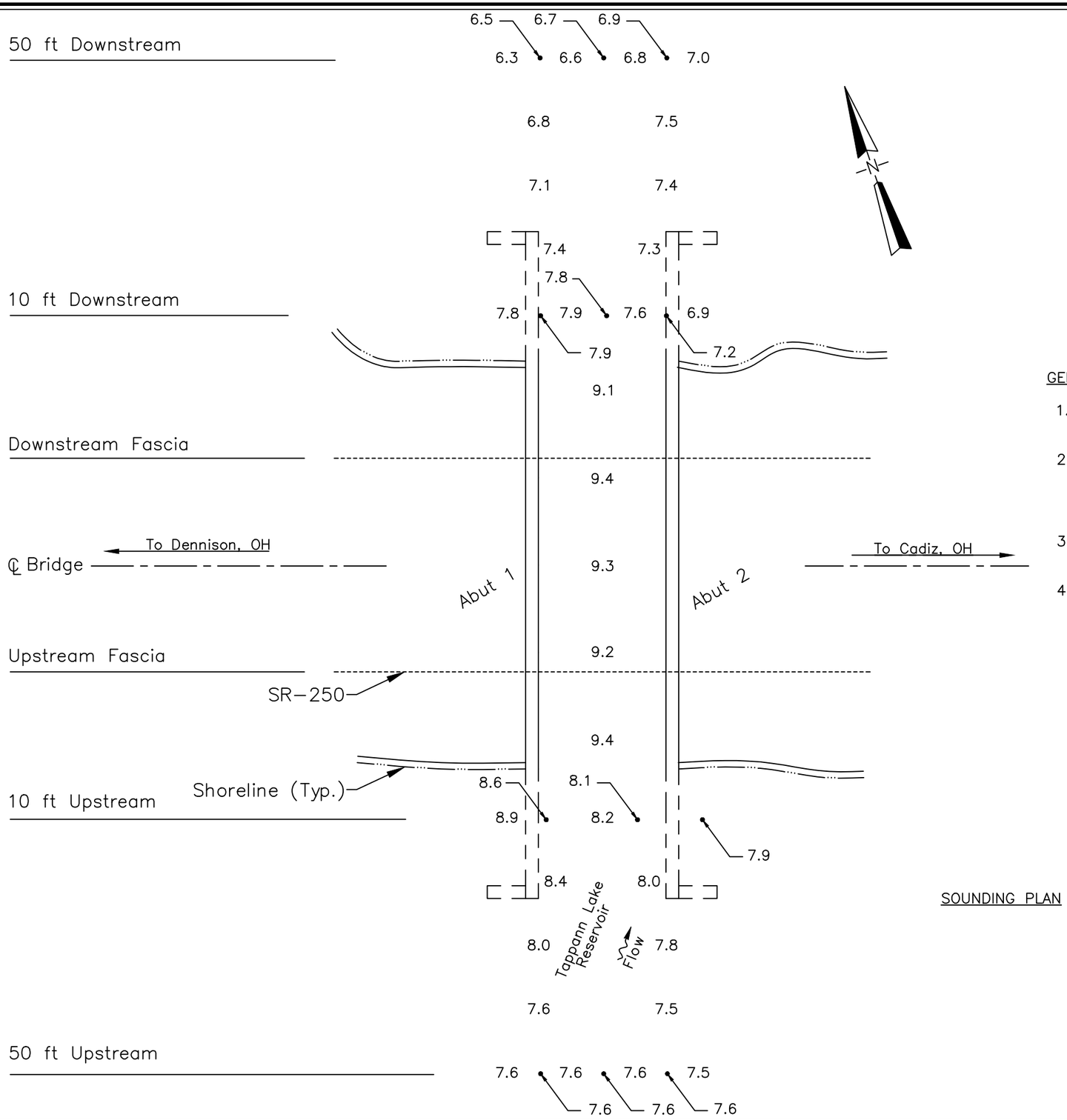

SOUNDING PLAN

GENERAL NOTES:

1. Abutments 1 and 2 were inspected underwater. Substructure units are labeled according to available record drawings.
2. At the time of inspection on August 2, 2017, the waterline was located approximately 9.8 ft below the top of slab at the mid-span on the south fascia. This corresponds with a waterline elevation of +898.40 ft.
3. Soundings indicate the water depth at the time of inspection and are measured in feet.
4. Soundings were taken parallel to the bridge at the upstream and downstream fascias, 50 ft upstream and downstream of the fascias, at 1/4 point intervals between the substructure units, and at 10 ft intervals in-line with the piers upstream and downstream up to 50 ft.

LEGEND

2.7	Sounding Depth from Waterline (ft)
	Timber Debris

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SR 250 OVER TAPPAN LAKE RESERVOIR
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SOUNDING PLAN
HARRISON COUNTY, OHIO

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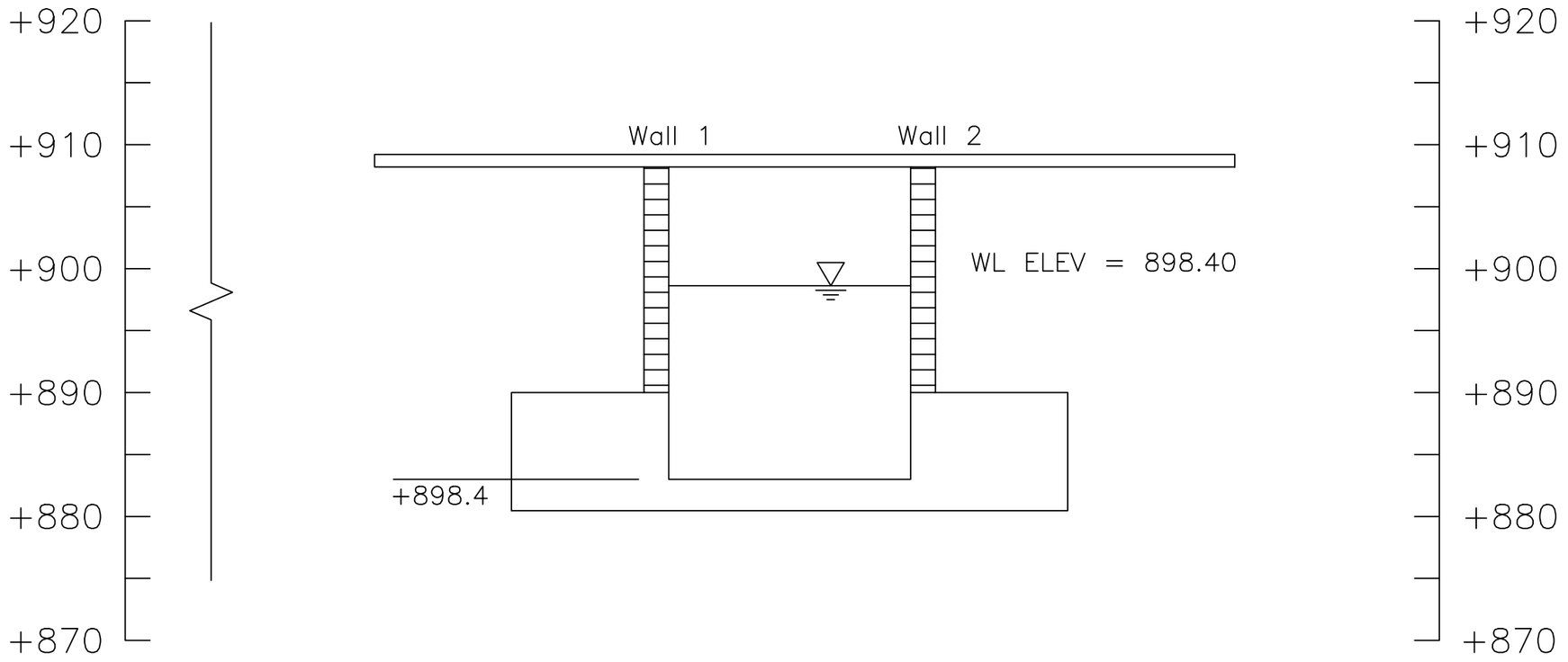

STRUCTURE CENTERLINE
CHANNEL CROSS SECTION
(SOUTH ELEVATION)

Note:

Footing elevations unknown due to unavailable record drawings.

LEGEND

----------	Upstream Fascia – 2017
– – – – –	Downstream Fascia – 2017
	Timber Debris
	Water Surface
+450	Elevation (ft)

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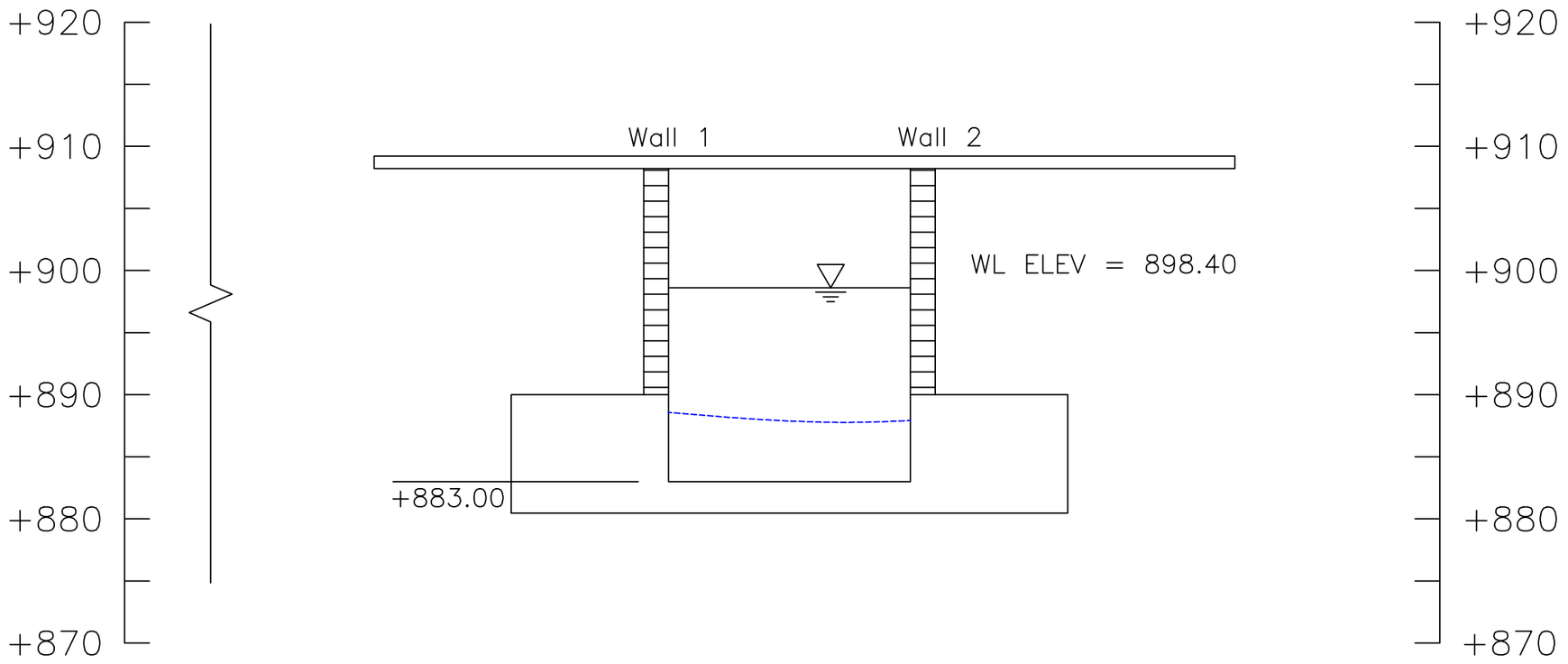

STRUCTURE CENTERLINE
CHANNEL CROSS SECTION
(SOUTH ELEVATION)

Note:

Footing elevations unknown due to unavailable record drawings.

LEGEND

----------	Upstream Fascia – 2017
– – – – –	Downstream Fascia – 2017
	Timber Debris
	Water Surface
+450	Elevation (ft)

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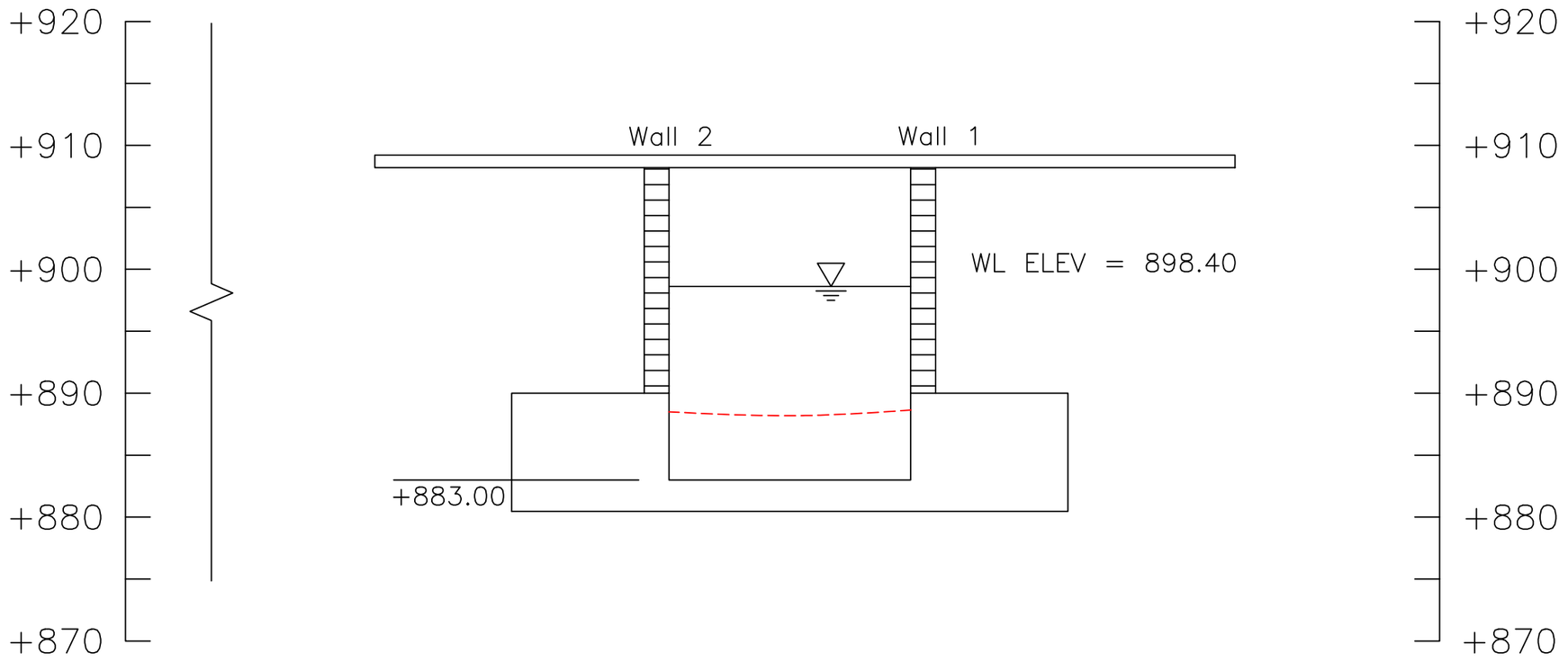

STRUCTURE CENTERLINE
CHANNEL CROSS SECTION
(NORTH ELEVATION)

Note:

Footing elevations unknown due to unavailable record drawings.

LEGEND

---------	Upstream Fascia – 2017
– – – – –	Downstream Fascia – 2017
	Timber Debris
	Water Surface
+450	Elevation (ft)

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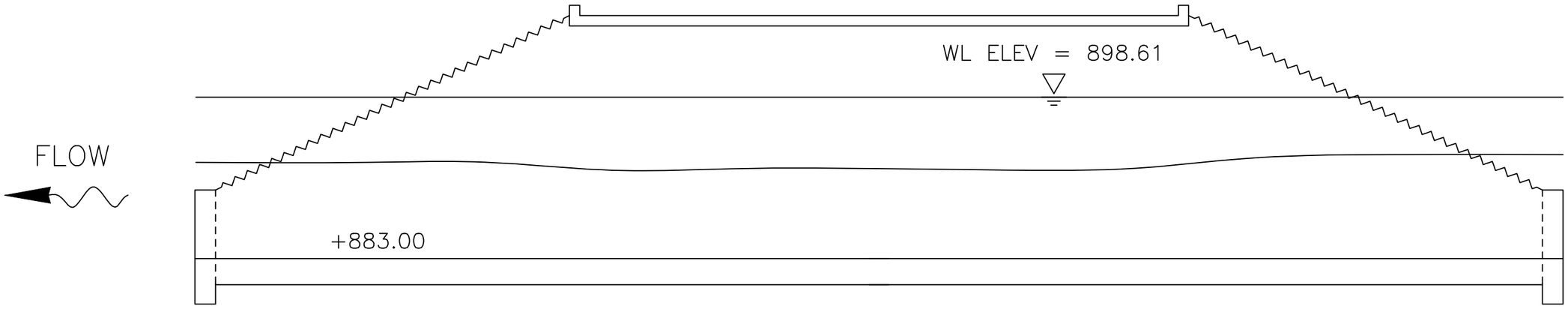

WALL 1
WEST ELEVATION

INSPECTION NOTES:

1. The channel bottom material consisted of soft silt.
2. The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep.

LEGEND

−2.7	Sounding Depth from Waterline (ft)
———	Approximate Channel Bottom – 2017
	Timber Debris
	Water Surface

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PIER 1
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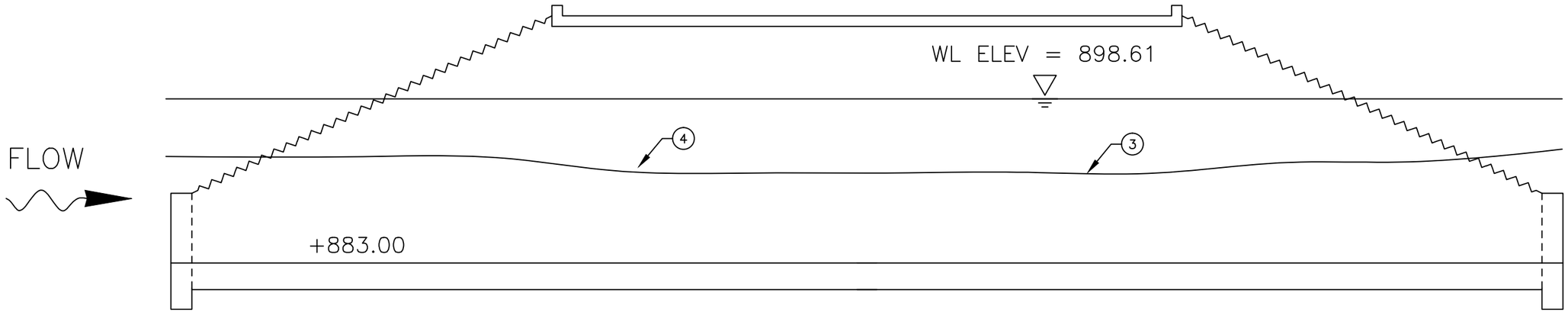

WALL 2
EAST ELEVATION

INSPECTION NOTES:

1. The channel bottom material consisted of soft silt.
2. The submerged portions of the abutment exhibited light marine growth and light scaling up to ⅛ in deep with isolated areas having scaling up to ¼ in deep.
3. Construction joint near south fascia is 1 in wide at channel bottom.
4. Construction joint near north fascia is ½ in wide at channel bottom.

LEGEND

−2.7	Sounding Depth from Waterline (ft)
———	Approximate Channel Bottom – 2017
	Timber Debris
	Water Surface

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EXHIBIT 2 – INSPECTION PHOTOGRAPHS



Photograph No. 1: Overall View of Structure No. HAS-250-0934, Looking North.



Photograph No. 2: Overall View of Structure No. HAS-250-0934, Looking South.



Photograph No. 3: View of the SW Embankment Upstream of the Structure, Looking NW.



Photograph No. 4: View of the SW Embankment at the Structure, Looking NE.



Photograph No. 5: View of the NW Embankment Downstream of the Structure, Looking SW.



Photograph No. 6: View of the NE Embankment Upstream of the Structure, Looking SE.



Photograph No. 7: View of the East Face of Abutment 1, Looking NW.



Photograph No. 8: View of the West Face of Abutment 2 , Looking NE.



Photograph No. 9: View of the Typical Concrete Condition at the Waterline on the West Face of Abutment 2, Looking East.