







UNDERWATER BRIDGE INSPECTION REPORT

STRUCTURE NO. SFN7903936
(TUS-77-3496 SB)
I-77 OVER TUSCARAWAS RIVER
TUSCARAWAS COUNTY, OH
DISTRICT 11

June 2022

Prepared for:

STATE OF OHIO DEPARTMENT OF TRANSPORTATION

STATE OF OHIO
MATTHEW ROGERS
PE.87504
REGISTERED PROFESSIONAL ENGINEER

Prepared by:

COLLINS ENGINEERS INC.

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EXECUTIVE SUMMARY

Project: ODOT D11 Underwater Bridge Inspections - 2022.

Purpose of Project: To perform a detailed visual and tactile underwater investigation of bridges for the Ohio Department of Transportation, District 11.

Inspection Team: Team Leader – Matthew Rogers, P.E. – Collins Engineers, Inc.
Team Member – Donald Lort, E.I.T. – Collins Engineers, Inc.
Team Member – Isaiha Easley – Collins Engineers, Inc.

Inspection Date(s): June 28, 2022

Water Visibility:	5 ft	***Water Velocity:***	1 ft/s
Water Temperature:	70 °F	***Weather:***	Sunny – 75 °F
Waterline Elevation:	889.5 ft	***Type of Boat:***	10 ft Kayak
High-water Elevation:	895.1 ft		
Coordinates:	40.663105°N, 81.438567°W		
Access Location:	North Embankment 50 ft downstream		
Dive Mode:	Wading		
Waterline Reference:	30.5 ft below the top of parapet at the upstream nose of Pier 2.		
Maximum Depth at SSU:	2.5 ft – South Face of Pier 3		
Shoreline Conditions:	The north and south shorelines consisted of moderately to heavily vegetated mild slopes with vertical cut banks up to 6 ft and multiple trees leaning toward the channel.		

Summary of Findings:

- **Pier 2:**
  - Scattered voids from poorly consolidated concrete measuring up to 1/2 in. deep were observed throughout the pier.
  - Moderate timber debris accumulation consisting of logs and trees up to 12 in. diameter was observed on the upstream nose of the pier from the channel bottom to 3 ft above the waterline.
- **Pier 3:**
  - Scattered voids from poorly consolidated concrete measuring up to 1/2 in. deep were observed throughout the pier.

Summary of Recommendations:

- Monitor timber debris accumulation at Pier 2.

Underwater Inspection Coding:

NBI Ratings:

Item	Description	Coding	Condition
B.C.03	Substructure (Overall)	7 – Good Condition	
B.C.09	Channel Cond. Rating	6 – Satisfactory Condition	Moderate Erosion, Cut Banks, Timber Debris Accumulation
B.C.10	Channel Protection Cond.	N – Not Applicable	
B.C.11	Scour Cond. Rating	8 – Above Foundation Limits	Stable (Inspector Recommended)
B.C.15	Underwater Cond. Rating	7 – Good Condition	
B.IE.03	Insp. Date	June 28, 2022	
B.IE.05	UW Insp. Interval	60 Months	

AASHTO National Bridge Element (NBE) Ratings:

Element #	Description	Units	Total	Condition State			
				1	2	3	4
210	Reinforced Concrete Pier Wall	LF	35	35	0	0	0

Note: Ratings were developed using the FHWA Specifications for the National Bridge Inventory. The recommended ratings consider inspected elements located within the waterway and conditions existing below the water surface only. Additional consideration is necessary for the assignment of overall condition ratings for this bridge.

1.0 INTRODUCTION

1.1 Purpose and Scope

This report consists of the results of a detailed underwater investigation performed at the I-77 Bridge over Tuscarawas River in Tuscarawas County, OH. Collins Engineers, Inc. (Collins) conducted the underwater investigation for the Ohio Department of Transportation (ODOT), District 11 on June 28, 2022. The primary purpose of the investigation was as follows:

- Determine the condition of the substructure components located in the water at the time of the inspection from the waterline to the channel bottom.
- Obtain channel bottom depth measurements along the bridge fascias, upstream and downstream of the bridge, and around the submerged substructure units.
- Determine the condition of the shorelines in the vicinity of the structure.
- Obtain photographs of the bridge and any significant defects.

In addition, a brief inspection was made of areas that could be submerged during periods of high water. The following report includes a description of the structure, the method of investigation, a description of existing conditions, an evaluation and recommendations based on the conditions, inspection figures, and photographs.

1.2 General Description of the Structure

Structure No. SFN7903936 spans 238.6 ft, carrying I-77 over Tuscarawas River and is approximately 44 ft wide. The bridge superstructure is constructed of three spans, each composed of five steel girders. The roadway orientation of the longitudinal axis of the bridge is south to north. The substructure units are labeled as Abutments 1 and 4 and Piers 2 and 3. Previous inspection reports were available at the time of the inspection. Refer to Figure 1 in Exhibit 1 for a Location Map of the bridge. Refer to Photographs 1 through 4 in Exhibit 2 for overall views of the bridge.

1.3 Method of Investigation

A detailed field inspection was conducted to determine the physical condition of the submerged bridge substructure units from the waterline to the channel bottom. A brief visual examination of the substructure units above the waterline was also made.

A three-person team consisting of professional engineer-diver and team leader (Matthew O. Rogers, P.E.), engineer-diver (Donald Lort, E.I.T.), and technician-diver (Isaiha Easley) conducted the underwater inspection. The inspection was conducted through wading. During the inspection, the inspectors worked from a boat and a note taker on the shore recorded the inspection notes.

The underwater inspection consisted of a visual and tactile examination of the accessible surfaces of the substructure units from the waterline to the channel bottom with particular attention given to any observed areas of deterioration or apparent distress. Approximately 10 percent of the total area on the underwater surfaces of the substructure units was cleaned so that the condition could be more closely examined. Photographs were taken to document the general conditions and observed deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining were noted.

Channel bottom soundings were performed utilizing a telescoping survey rod and digital fathometer. Soundings were taken parallel to the bridge at the upstream and downstream fascias, at quarter intervals between the substructure units, and at 10 ft intervals in-line with the piers upstream and downstream up to 50 ft, and the waterline was referenced to a known elevation on the bridge. A sounding plan was developed using the soundings and approximate location of the shorelines. Refer to Figures 2 through 5 in Exhibit 1 for the sounding plan and channel cross sections that show the channel limits and water depths around the structure.

2.0 EXISTING CONDITIONS

2.1 General Conditions

At the time of the inspection, the waterline of Tuscarawas River was located approximately 30.5 ft below the top of parapet at the upstream nose of Pier 2, which corresponds to a waterline elevation of 889.5 ft. During the inspection, the waterway was flowing west to east below the structure at an approximate rate of 1 ft/s. The approximate high-water elevation was observed to be 895.1 ft.

The bridge pier skew was consistent with the channel alignment and does not require attention at this time. The north and south shorelines consisted of moderately to heavily vegetated mild slopes with vertical cut banks up to 6 ft and multiple trees leaning toward the channel. Refer to Photographs 3 through 10 in Exhibit 2 for views of the shorelines near the structure.

2.2 Substructure Conditions

2.2.1 Pier 2

The channel bottom material around Pier 2 consisted of gravel, sand, and timber debris with approximately 6 in. probe rod penetration. Typical light marine growth algae and silt up to 1/8 in. deep was observed on the submerged portions of the pier. Scattered voids from poorly consolidated concrete measuring up to 1/2 in. deep were observed throughout the pier. Moderate timber debris accumulation consisting of logs and trees up to 12 in. diameter was observed on the upstream nose of the pier from the channel bottom to 3 ft above the waterline. Refer to Figure 6 in Exhibit 1 for detailed inspection notes of Pier 2. Refer to Photographs 12 through 15 in Exhibit 2 for views of Pier 2, timber debris, and typical concrete condition at the waterline.

2.2.2 Pier 3

The channel bottom material around Pier 3 consisted of silty sand with river gravel with approximately 18 in. probe rod penetration. Typical light marine growth algae and silt up to 1/8 in. deep was observed on the submerged portions of the pier. Scattered voids from poorly consolidated concrete measuring up to 1/2 in. deep were observed throughout the pier. Refer to Figure 7 in Exhibit 1 for detailed inspection notes of Pier 3. Refer to Photographs 16 through 17 in Exhibit 2 for views of Pier 3.

2.2.3 Channel Bottom and Scour Assessment

At the time of the inspection, the Tuscarawas River was experiencing normal flow and the channel bottom appeared stable for the type of material. The channel bottom was primarily gravel and sand surrounding the substructure units. No additional scour, undermining, or scour countermeasures were observed.

3.0 EVALUATION AND RECOMMENDATIONS

Overall, the submerged substructure units of Structure No. SFN7903936 were in good condition below water. A comparison of the soundings recorded during the previous inspection on July 18, 2017 and the soundings taken during this inspection revealed no significant change in the channel bottom profile in the vicinity of the structure. Although no channel deficiencies were observed, the channel bottom should continue to be monitored during future underwater inspections to verify that localized scour or overall channel degradation is not occurring and that the pier footings remain adequately embedded in the channel bottom.

The timber debris accumulation at Pier 2 did not significantly affect the channel flow, and as a result, does not require removal at this time. If the debris accumulation increases in size or density, it may be necessary to remove the debris to reduce excessive lateral loads on the pier, limit further debris accumulation, and reduce the likelihood of channel bottom degradation resulting from obstructed flow.

The voids associated with poor concrete consolidation at Piers 2 and 3 are not structural concerns at this time given their size compared to the overall pier size, and as a result, no repairs are recommended. This area should be monitored during future inspections for increasing extent or severity of the voids and exposure of reinforcing steel. If the extent or severity of the voids are observed to be increasing or reinforcing steel becomes exposed, it may be necessary to repair the area at that time.

It is recommended that the submerged substructure units of Structure No. SFN7903936 be next inspected underwater at an interval not to exceed 60 months, no later than June 28, 2027.

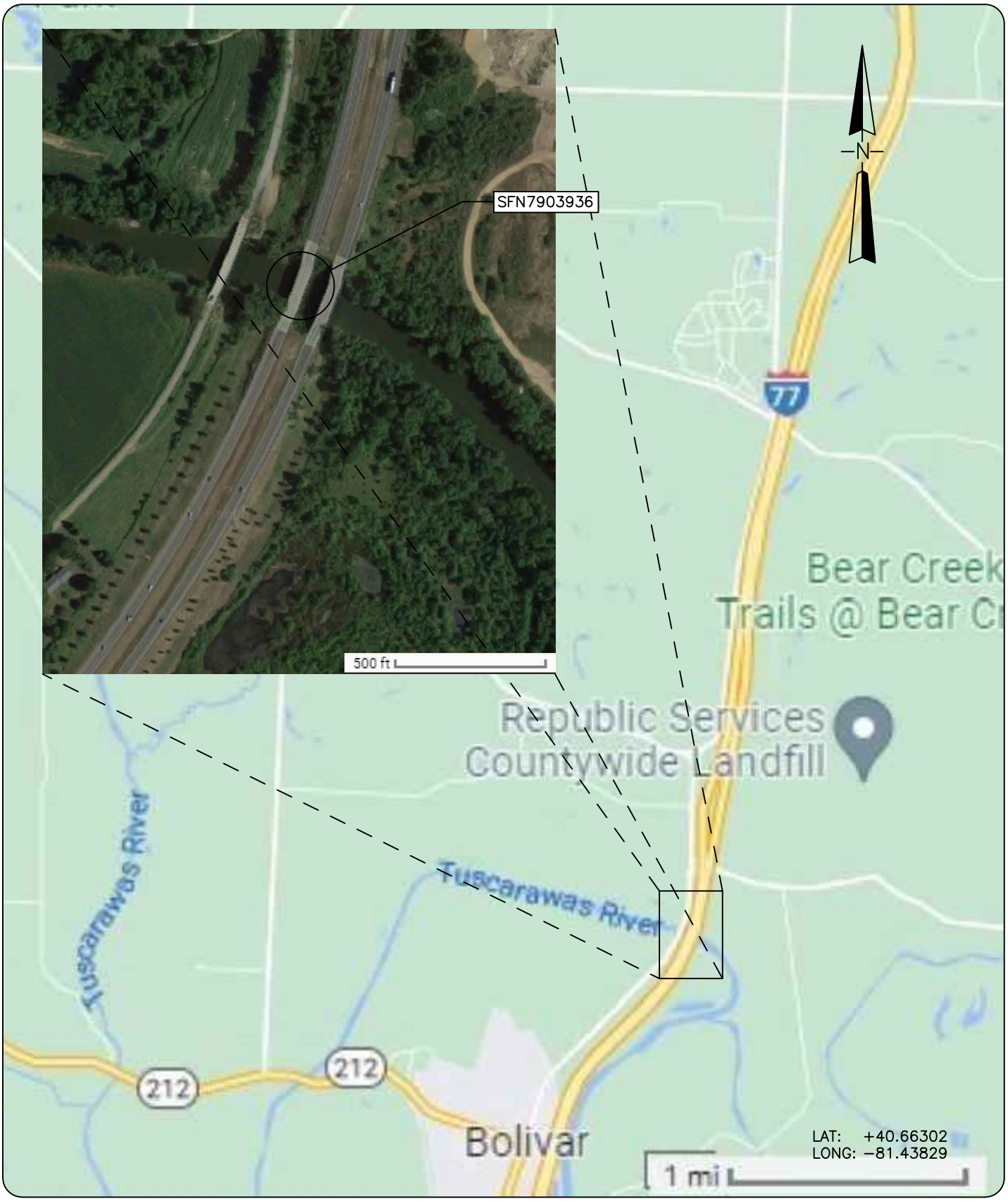
Respectfully Submitted,
COLLINS ENGINEERS, INC.

Matthew Rogers

Matthew Rogers, P.E.
Project Manager

Originated by:
Victor Valdivia, E.I.T.

EXHIBIT 1 – FIGURES



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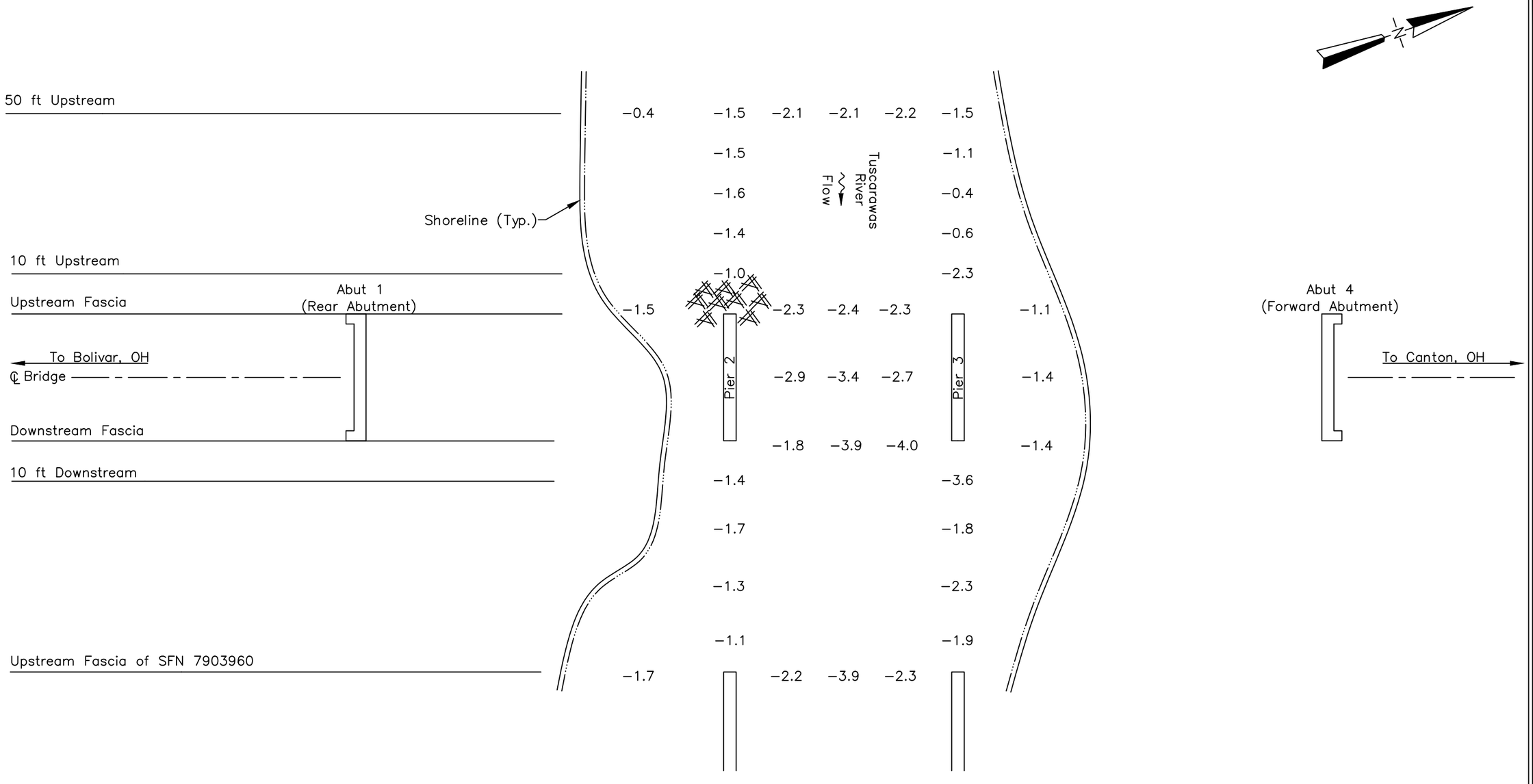
I-77 OVER TUSCARAWAS RIVER
STRUCTURE NO. SFN 7903936
LOCATION MAP

TUSCARAWAS COUNTY, OHIO

INSPECTED BY: MOR	CEI PROJECT: 55−13380.01
DRAWN BY: OPF	DATE: 28 JUN 2022
CHECKED BY: JMJ	SHEET NO: 1



SOUNDING PLAN

GENERAL NOTES:

1. Piers 2 and 3 were inspected underwater. Substructure units are labeled according to available record drawings.
2. At the time of inspection on June 28, 2022, the waterline was located approximately 30.5 ft below the top of parapet at the upstream nose of Pier 2. This corresponds with a waterline elevation of +889.5 ft.
3. Soundings indicate the water depth at the time of inspection and are measured in feet.
4. Soundings were taken parallel to the bridge at the upstream and downstream fascias, 50 ft upstream and downstream of the fascias, at 1/4 point intervals between the substructure units, and at 10 ft intervals in−line with the piers upstream and downstream up to 50 ft.

LEGEND

−2.7	Sounding Depth from Waterline (ft)
	Timber Debris

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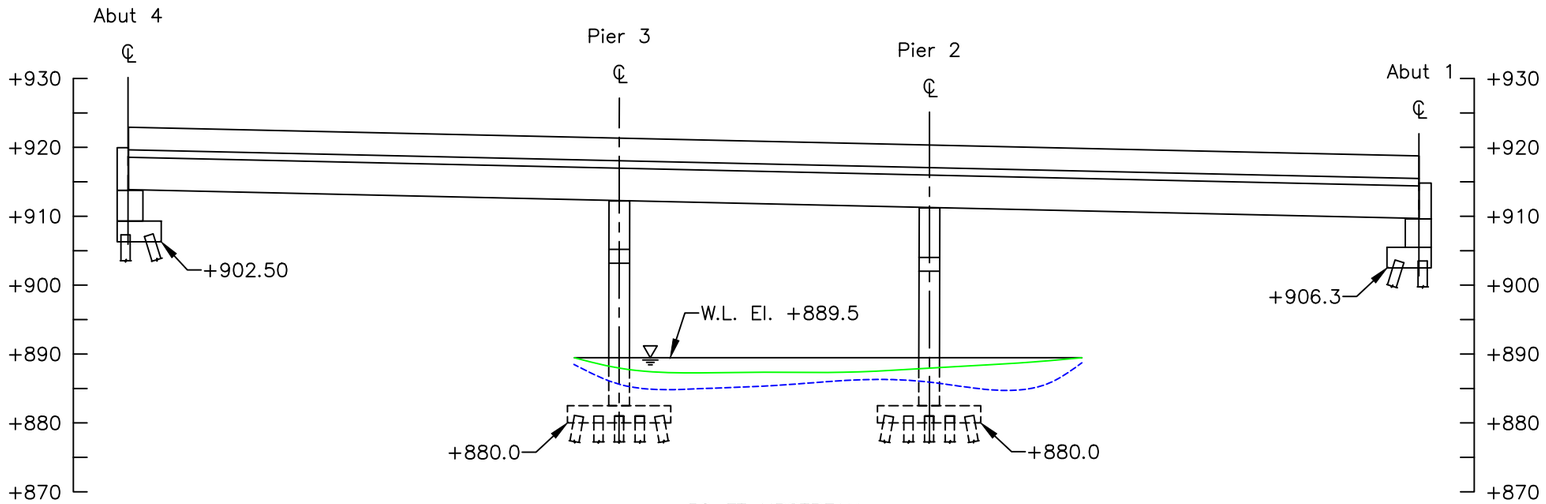
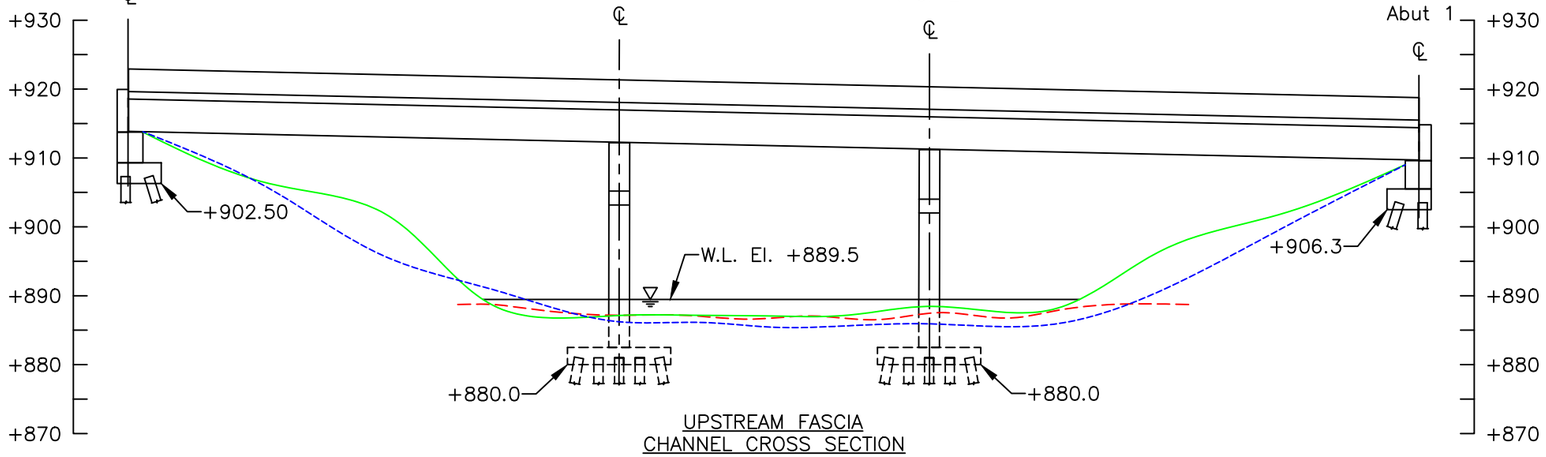
I-77 OVER TUSCARAWAS RIVER
STRUCTURE NO. SFN 7903936
SOUNDING PLAN
TUSCARAWAS COUNTY, OHIO

CEI PROJECT	55−13380.01
INSPECTED BY:	MOR
DRAWN BY:	OPF
CHECKED BY:	JMJ
DATE:	JUN 2022
SHEET NO:	2



50 FT UPSTREAM
CHANNEL CROSS SECTION
(LOOKING DOWNSTREAM)

Upstream Elevations (Looking Downstream)	
Location	Height (ft)
A4	8.5
1/4	16.3
1/2	20.2
3/4	26.4
P3	34.2
1/4	33.9
1/2	33.8
3/4	33.4
P2	31.9
1/4	32.0
1/2	22.9
3/4	17.6
A1	9.8



UPSTREAM FASCIA
CHANNEL CROSS SECTION
(LOOKING DOWNSTREAM)

LEGEND

- Approximate Channel Bottom – June 2022
- Approximate Channel Bottom – July 2017
- Approximate Channel Bottom – September 2012
- Timber Debris
- Water Surface
- +450 Elevation (ft)

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I-77 OVER TUSCARAWAS RIVER
STRUCTURE NO. SFN 7903936
CROSS SECTIONS
TUSCARAWAS COUNTY, OHIO

CEI PROJECT 55–13380.01
INSPECTED BY: MOR
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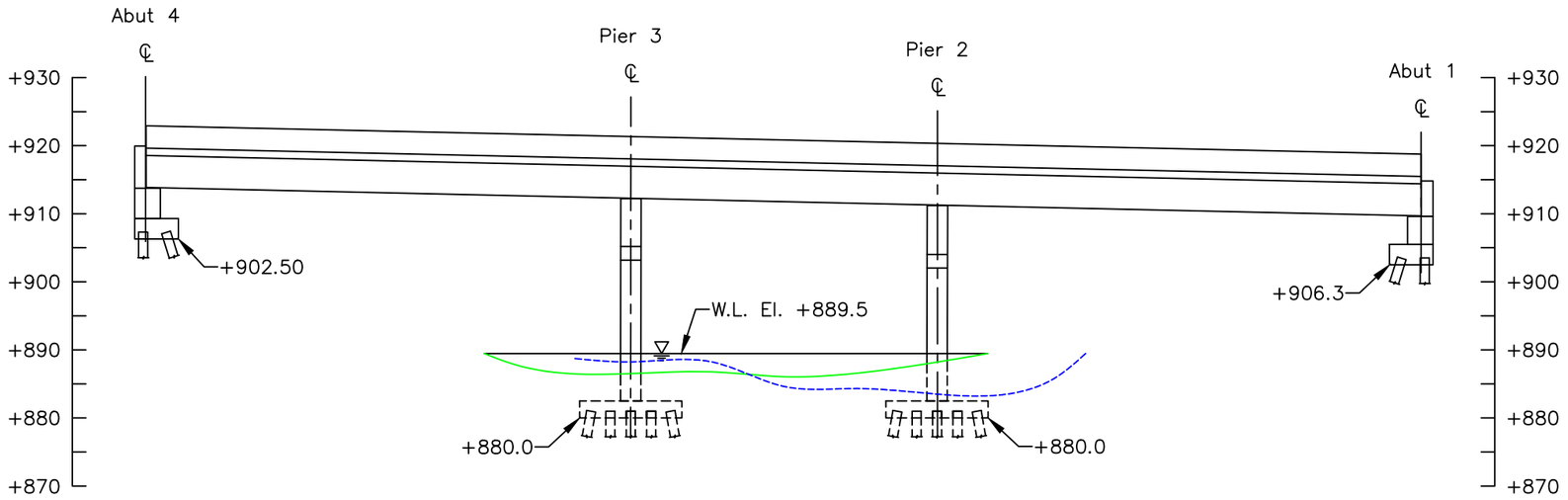
CHECKED BY: JMJ
DATE: JUN 2022
SHEET NO: 3



STRUCTURE CENTERLINE
CHANNEL CROSS SECTION
(LOOKING DOWNSTREAM)

LEGEND

———	Approximate Channel Bottom – June 2022
- - - - -	Approximate Channel Bottom – July 2017
– – – – –	Approximate Channel Bottom – September 2012
	Timber Debris
	Water Surface
+450	Elevation (ft)

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I-77 OVER TUSCARAWAS RIVER
STRUCTURE NO. SFN 7903936
CROSS SECTIONS
TUSCARAWAS COUNTY, OHIO

CEI PROJECT	55–13380.01
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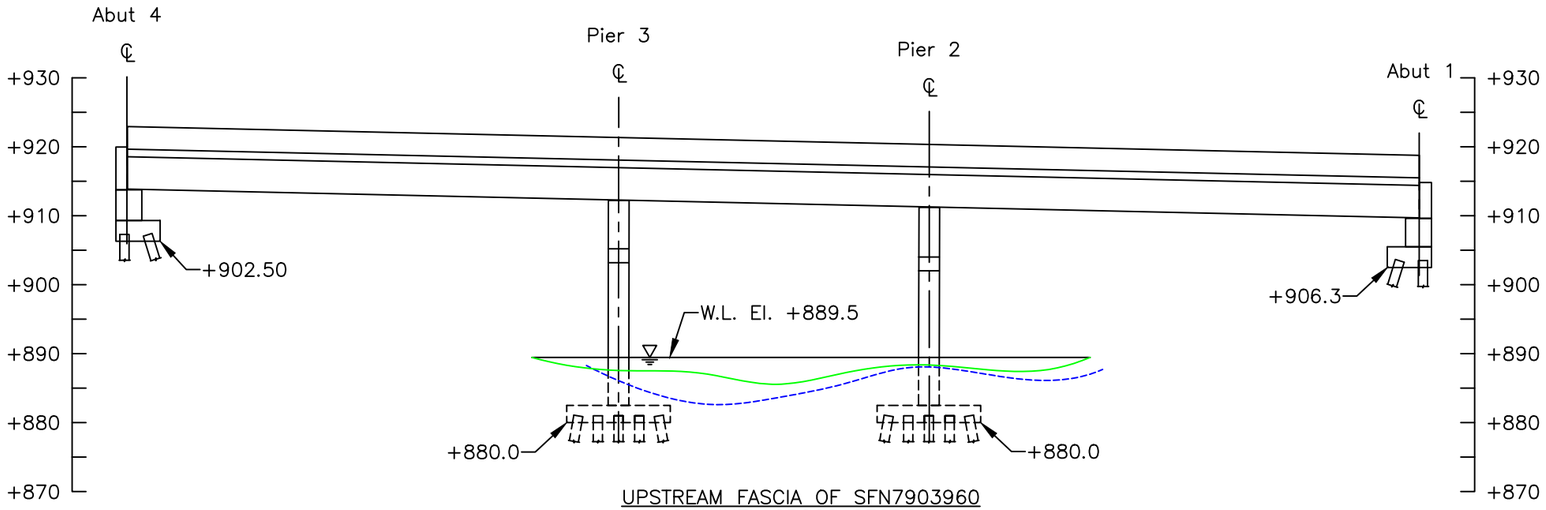
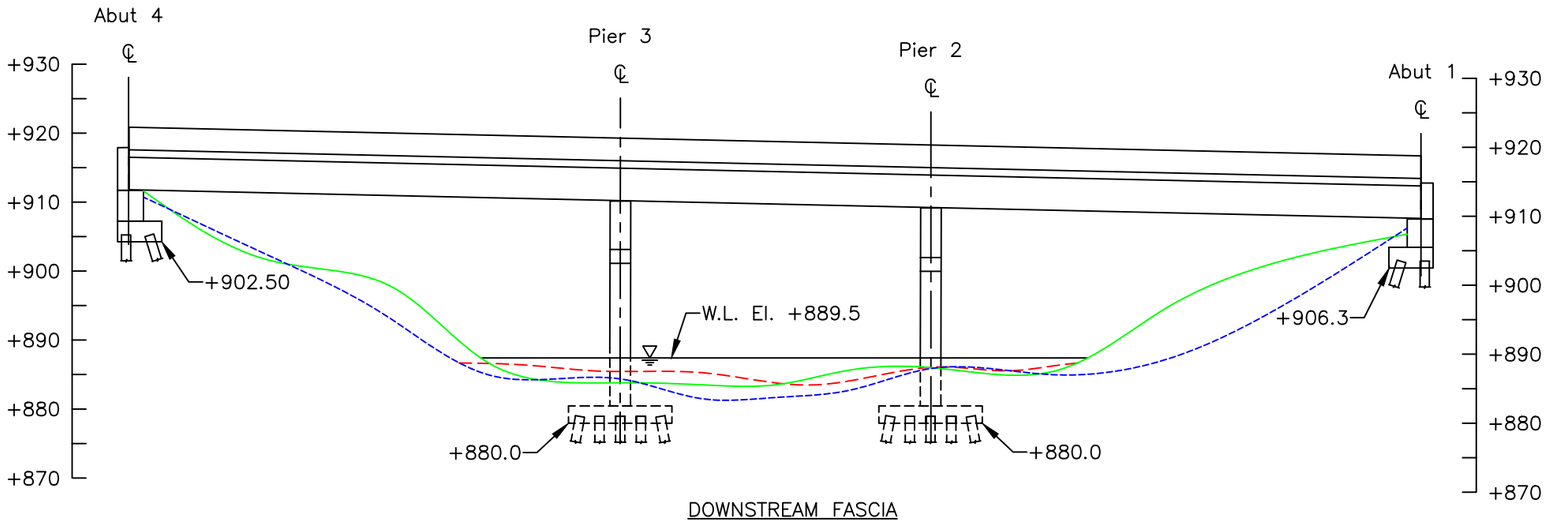
SHEET NO:	4



UPSTREAM FASCIA OF SFN7903960
CHANNEL CROSS SECTION
(LOOKING DOWNSTREAM)

Downstream Elevations (Looking Downstream)	
Location	Height (ft)
A4	9.2
1/4	19.0
1/2	21.9
3/4	33.7
P3	35.5
1/4	35.4
1/2	35.3
3/4	32.9
P2	32.3
1/4	32.4
1/2	23.0
3/4	18.9
A1	11.4



DOWNSTREAM FASCIA
CHANNEL CROSS SECTION
(LOOKING DOWNSTREAM)

LEGEND

- Approximate Channel Bottom – June 2022
- Approximate Channel Bottom – July 2017
- Approximate Channel Bottom – September 2012
- Timber Debris
- Water Surface
- +450 Elevation (ft)

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CROSS SECTIONS
TUSCARAWAS COUNTY, OHIO

CEI PROJECT 55–13380.01
INSPECTED BY: MOR
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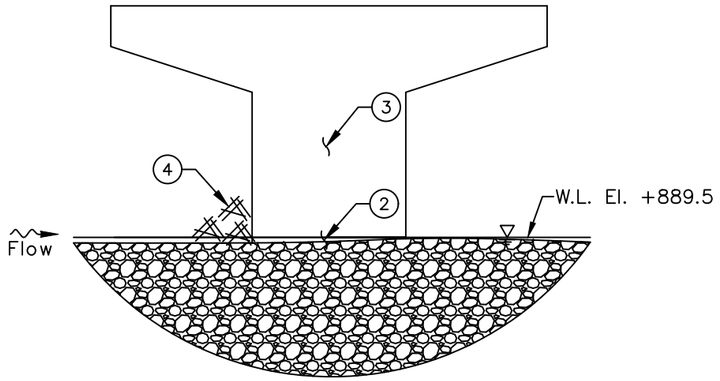
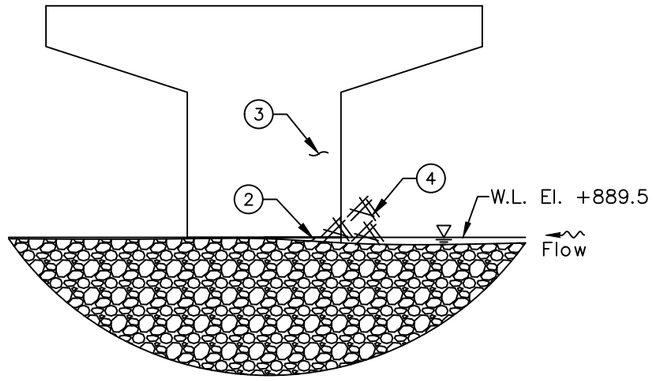
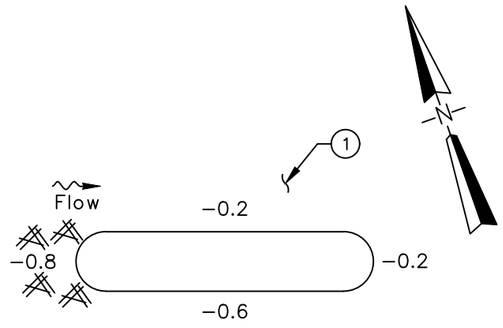
CHECKED BY: JMJ
DATE: JUN 2022
SHEET NO: 5



SOUTH ELEVATION
(LOOKING NORTH)



NORTH ELEVATION
(LOOKING SOUTH)



INSPECTION NOTES:

1. The channel bottom material consisted of gravel, sand, and timber debris with approximately 6 in. probe rod penetration.
2. Typical light marine growth algae and silt up to 1/8 in. deep was observed on the submerged portions of the pier.
3. Scattered voids from poorly consolidated concrete measuring up to 1/2 in. deep were observed throughout the pier.
4. Moderate timber debris accumulation consisting of logs and trees up to 12 in. diameter was observed on the upstream nose of the pier from the channel to 3 ft above the waterline.

LEGEND

−2.7	Sounding Depth from Waterline (ft)
———	Approximate Channel Bottom – June 2022
	Timber Debris
	Water Surface

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I-77 OVER TUSCARAWAS RIVER
STRUCTURE NO. SFN 7903936
PIER 2
TUSCARAWAS COUNTY, OHIO

CEI PROJECT 55–13380.01
INSPECTED BY: MOR
DRAWN BY: OPF
CHECKED BY: JMJ
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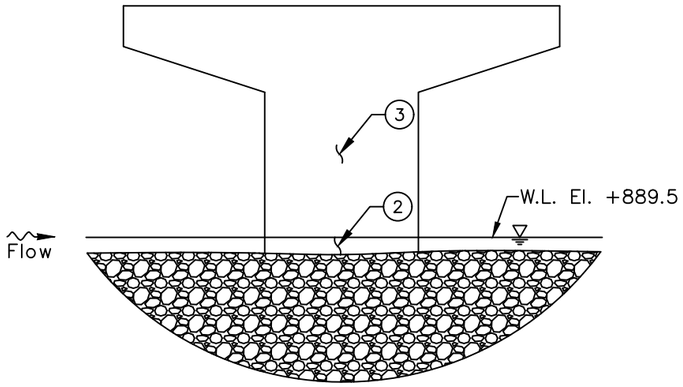
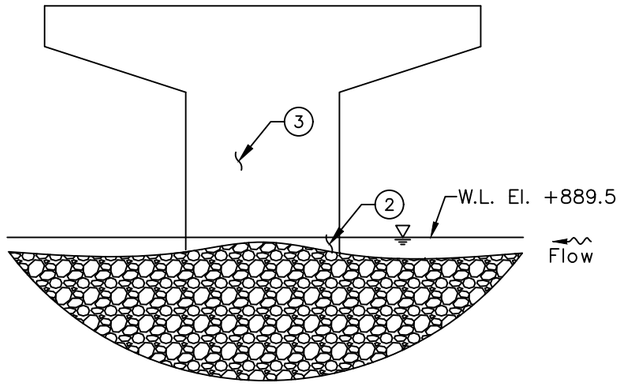
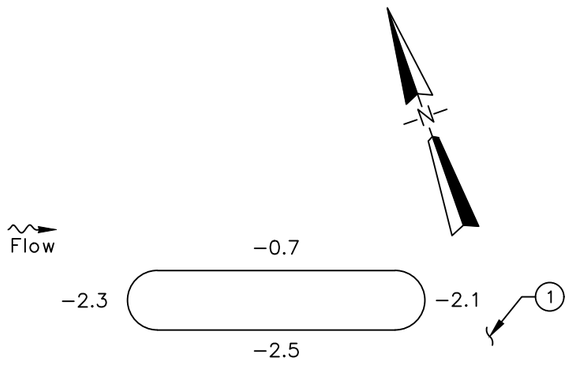
SHEET NO: 6



SOUTH ELEVATION
(LOOKING NORTH)



NORTH ELEVATION
(LOOKING SOUTH)



INSPECTION NOTES:

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2. Typical light marine growth algae and silt up to 1/8 in. deep was observed on the submerged portions of the pier.
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LEGEND

−2.7	Sounding Depth from Waterline (ft)
———	Approximate Channel Bottom – June 2022
	Timber Debris
	Water Surface

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PIER 3
TUSCARAWAS COUNTY, OHIO

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SHEET NO:	7

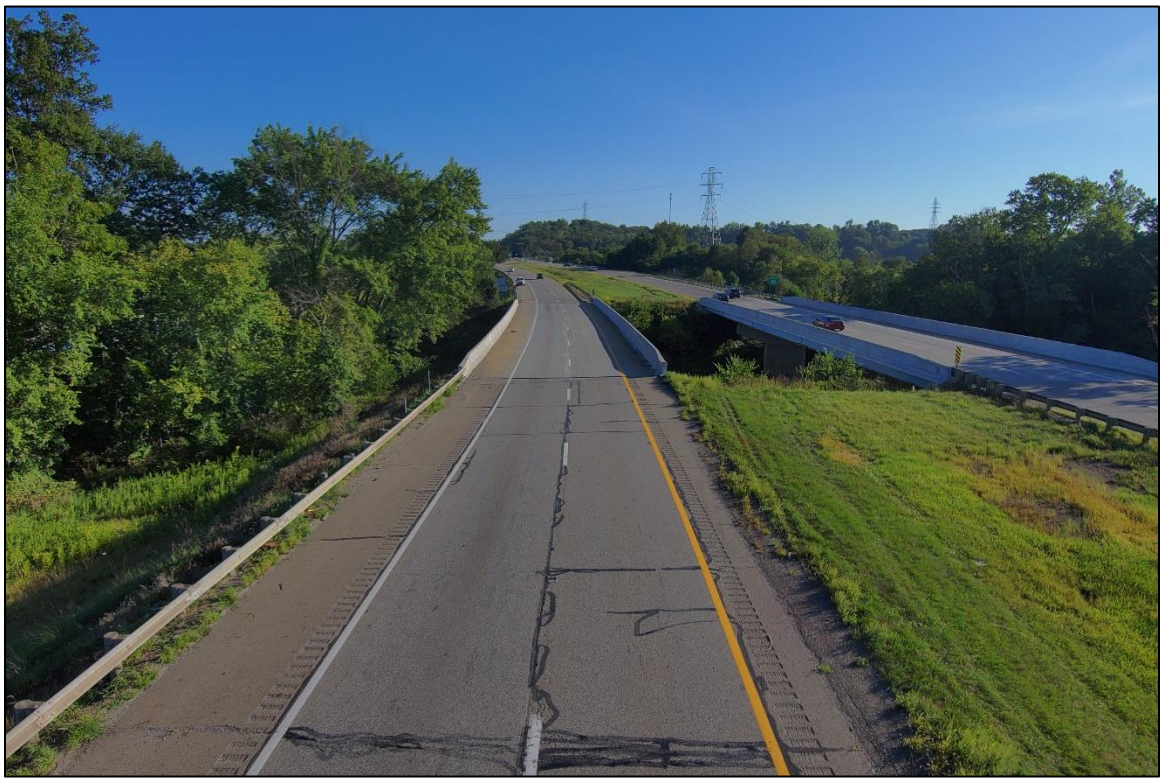
EXHIBIT 2 – INSPECTION PHOTOGRAPHS



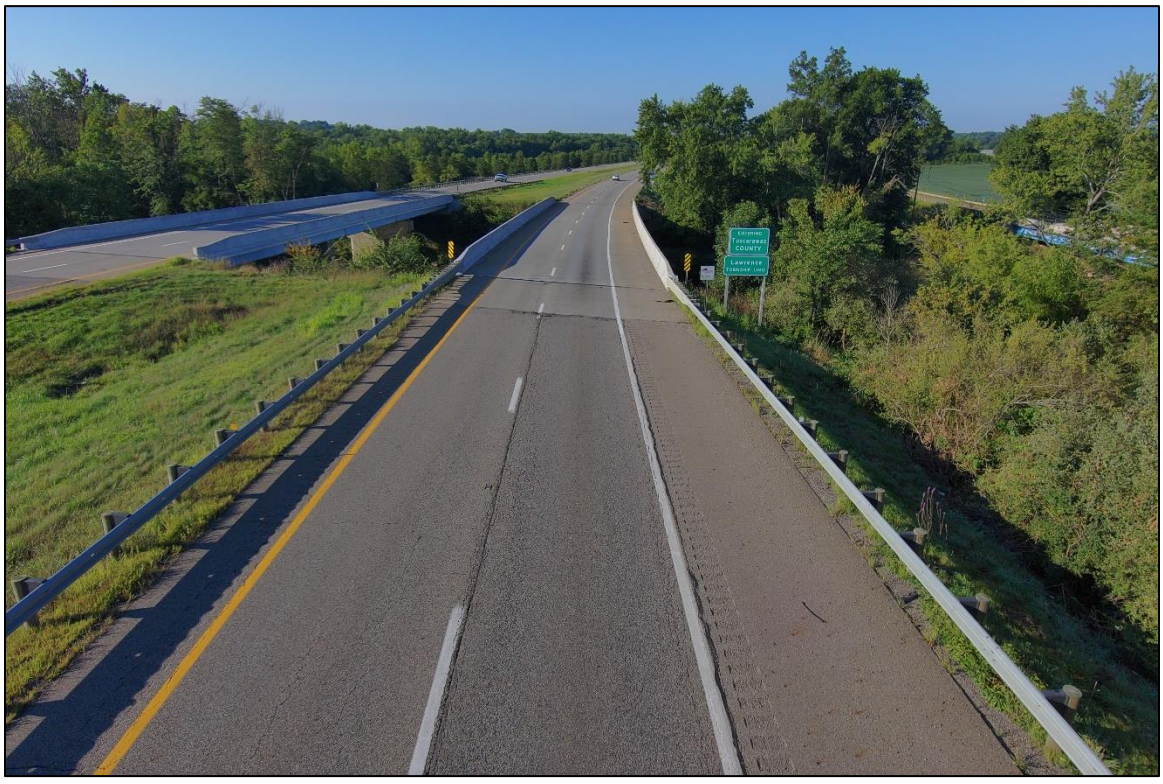
Photograph No. 1: Overall View of Structure No. SFN7903936, Looking East.



Photograph No. 2: Overall View of Structure No. SFN7903936, Looking West.



Photograph No. 3: South Approach of Structure No. SFN7903936, Looking North.



Photograph No. 4: North Approach of Structure No. SFN7903936, Looking South.



Photograph No. 5: View of the North Embankment Upstream of the Structure, Looking Northwest.



Photograph No. 6: View of the North Embankment at the Structure, Looking Northwest.



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Photograph No. 12: View of the South Face of Pier 2, Looking Northeast.



Photograph No. 13: View of the North Face of Pier 2, Looking South.



Photograph No. 14: View of Pier 2 with Timber Debris, Looking Southeast.



Photograph No. 15: View of the Typical Concrete Condition at the Waterline at the Downstream Nose of Pier 2, Looking Northwest.



Photograph No. 16: View of the South Face of Pier 3, Looking North.



Photograph No. 17: View of the North Face of Pier 3, Looking South.

EXHIBIT 3 – DIVE PROCEDURE CHECKLIST

Underwater Dive Inspection Procedure Checklist

Acceptable written procedures communicate to the next dive team what is necessary to ensure a safe and successful inspection. Each bridge requiring underwater dive techniques must have a unique written inspection procedure. The prior inspection report does not suffice for the required procedures. It is valuable to review the last inspection notes, but they do not serve the same purpose as a stand-alone inspection procedure.

This document shall be completed for all underwater dive inspections. This document shall be reviewed prior to performing the field work and it shall be updated when necessary.

I. Bridge Identification

a. Agency with Inspection Responsibility: ODOT DISTRICT 11

Dive Frequency: 60 months

SFN: 7903936 Bridge Number (County-Route-SLM-SD): TUS-77-3495 SB

Superstructure Type Main Span Type: STEEL GIRDER

Approach Span: STEEL GIRDER

Substructure Type Abutment Type: REINFORCED CONCRETE

Pier Type: REINFORCED CONCRETE

Total Pier Count: 2

Total Pier Count in water: 2

Foundations: CAST-IN-PLACE REINFORCED CONCRETE PILES

Feature Intersected TUSCARAWAS RIVER

b. Photographs



Endview



Elevation



Underside

II. Office and Field Assessment

Prior to the inspection, obtain and review copies of the previous underwater inspection reports, routine inspection reports, scour and hydraulic information, and design plans in preparation of the inspection. Divers should pay particular attention given to any observed areas of deterioration, the channel conditions and factors that may accelerate material deterioration. Changes shall be noted in the inspection procedure. Site conditions should be reviewed prior to diving.

a. Channel Conditions

____Waterway features

____Rapid stream flows,

__X_Significant debris accumulation

____Constricted waterway openings

____Soft or unstable streambeds

____Meandering channels

____Other which may promote scour and undermining of substructure elements

____Navigable Waterway

____Flow Controls

b. Anticipated Water conditions which may affect the inspection

____Cold Water (Apprx. Temp____)

____Black water

____Rapid stream flows

____Near military facility

____Tribal fishing

____Water quality

____History of Log jams

c. Identify factors that may accelerate the deterioration of the bridge elements:

____Highly corrosive water

____Unprotected steel members

____Other

Risk Factor Narrative:

III. **Contacts Prior to Work**

Dr. Waseem Khalifa, Ph. D., P.E.

District Bridge Engineer

2201 Reiser Avenue SE

New Philadelphia, OH 44663

(330) 308-7873

Point of contact for immediate action such as closing the bridge due to findings)

Contact Bridge Owner _____14_____ (number) days before the proposed underwater inspection.

Special contracting and scheduling procedures prior to inspection, include recommended lead time

Entity	Contact Name and Title	Contact Phone	Lead Time
Coast Guard	N/A	N/A	N/A
Property Owner	N/A	N/A	N/A
Access Equipment	N/A	N/A	N/A
Lake or River draw-down	N/A	N/A	N/A
Canal dry time	N/A	N/A	N/A
Tree removal	N/A	N/A	N/A
Other:	N/A	N/A	N/A
Other:	N/A	N/A	N/A

IV. **Dive Team Shall Include the Following:**

Dive Team Narrative:

The structure shall be investigated using a three-member dive team: one dive supervisor to monitor communications and air supply while taking notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.

Example: The Bridge shall be investigated using a three-member dive team: one supervisor to monitor rack box and take notes, one diver, and one tender/standby diver. There shall be one NBIS Team Leader onsite at all times.

V. Site Information

Navigable waterway:	Y / **N**	Anticipated current	1 ft
If Yes, waterway river point	_____	Scour Critical (item 113):	8
Anticipated water visibility depth	5 ft	POA in place: Y/**N**	
Anticipated Dive depth	2.5 ft	Scour Monitoring devices present: Y/**N**	

Verify the Scope of Services when work is contracted for the procedure for underwater elements that are not in water during an inspection.

Site Information Narrative:

The underwater inspection consists of a visual and tactile examination of the accessible surfaces of the substructure items in water. Additional items should reference the scope of services in the contract. For reference the following items are in water:

Item	Number of Units	Level of Inspection (1, 2 or 3) with Commentary
Piers and Number of Columns	2	100% Level 1 10% Level 2
Abutment		
Culvert		
Scour Countermeasures		
Fenders or Dolphins		

Photographs should be taken, if water clarity permits, for typical conditions, conditions that have changed since last inspection and significant or noteworthy deficiencies. The type of channel bottom material, the presence or extent of scour, the presence or extent of riprap, the presence or extent of drift and debris, and the location of any foundation exposure or undermining shall be quantified. Include depth, length, height and location of deficiencies.

VI. **Equipment and Field Logistics**

a. The inspection should be conducted using:

__X_Chest waders

____Hip waders

____Diving equipment

____SCUBA (Note that ADCI Consensus Standards require communication systems be employed for both SCUBA and Surface-Supplied (whether air or mixed-gas) dive modes)

____SCUBA with communication

____Surface Supplied with communication

b. The channel bottom should be sounded utilizing

__X_Digital fathometer

__X_Telescoping survey rod

____ acoustic imaging

c. During the inspection, the divers should work from

____Shore

__X_Boat

____Either

The note taker should work alongside the dive team.

d. Access to the waterway should be obtained from the shore (north bank, southwest quadrant, driveway 30 yards north etc.)

NORTH EMBANKMENT 50 FT DOWNSTREAM OF STRUCTURE_____

_____

_____

e. The maximum depth of the channel is typically measured 2.5 FT feet from THE SOUTH FACE OF PIER 3.

Reference Datum: 30.5 FT BELOW THE TOP OF PARAPET THE UPSTREAM NOSE OF PIER 2.

Soundings should be dictated by the scope of work. When not detailed in the scope they should be repeated from the previous soundings. If neither exist then they need to be taken in a grid pattern between substructure units 100’ upstream and 100’ downstream.

VII. Inspection Procedure History

Created: COLLINS ENGINEERS, INC. Date: 08/18/2022

Updated By: _____ Date: _____

Updated By: _____ Date: _____

Updated By: _____ Date: _____

Updated By: _____ Date: _____

Updated By: _____ Date: _____

VIII. Other Narrative Not Included In Previous Sections