

MR 509
Permit No. 11-2024-0128D

Office Use Only

State of Ohio
Department of Transportation
Permit

County or Jurisdiction BEL	
Rte	SR149
Log Pt	23.71
Acc Cat	

[1] Subject to all terms, conditions, and restrictions printed, written below and on the reverse side hereof, or attached,

Name: Love's Travel Stops & Country Stores, Inc.
Address: 10601 N. Pennsylvania Ave Oklahoma City OK 73120
Company Phone: 405-203-5900

is hereby granted a permit under Section 5515.01 and 5515.02 of Ohio Revised Code, and permission to perform work necessary in the manner described and at the location indicated in the following or attached to this permit.

Drive - Commercial - (see attached sheets)

Description of Work: Love's will construct a public access drive off of SR 149 for access to the proposed Love's Travel Stop development. In conjunction with the construction of the proposed access drive, a southbound right turn lane along SR 149 will be constructed for access onto existing Reco Rd. A dedicated right turn lane from SR 149 to the I-70 EB Entrance ramp will also be striped. A dedicated left turn lane SR 149 northbound for access to Reco Drive will be added. A two way left turn lane will be striped between Reco Drive and the proposed Love's driveway. A dedicated southbound right turn lane will be added for access to the proposed Love's driveway on SR 149. A dedicated left turn lane will be striped for access from SR 149 northbound to the proposed Love's driveway. It is anticipated that this construction will be started in June 2024 in conjunction with the Love's Travel Stop development and completed by May 2025.

[2] This permit shall be in the possession of employees /agents of permittee on site at all times who are in charge of the work and shall be shown, upon request, to any employee of the Department of Transportation.

Contact ODOT Representative 3 days before work begins, also contact ODOT Representative when work is completed for final inspection.

Failure to notify the ODOT Representative could result in work stoppage!

[3] No work authorized by this permit shall begin until the permittee has contacted and received instructions from

ODOT Representative	Dave Schafer
Phone	330-308-6510
Email Address:	David.Schafer@dot.ohio.gov

(Authorized ODOT Employee)

NOTE: Any work performed by the permittee may be stopped if this requirement is not met.

[4] Prior to any excavation in the highway right-of-way, the Ohio811, <https://www.oups.org/excavators>, must be contacted in accordance with ORC Section 3781.25 to 3781.32. Ohio811 can be reached at 1-800-362-2764 or 811.

[5] If your utility is above ground in any way, you must mark your utility with a fluorescent colored marker that corresponds with the universal OUPS color code. The marker must be no shorter than six feet in height and you must maintain the marker. Guide wires must be marked a fluorescent yellow. Failure to mark as described, will result in the Department of Transportation being held harmless and no reimbursement for damage to your property.

[6] All work requiring persons or vehicles within ODOT right of way shall comply with all applicable requirements of the Ohio Manual of Uniform Traffic Control Devices and Item 614 (Maintaining Traffic) of the Construction and Material Specifications, latest editions. Failure to comply with these requirements will be cause for immediate revocation or suspension of the permit until the proper traffic control devices have been provided.

[7] The permittee accepts the conditions, terms, and requirements printed, written on, or attached to this permit and understands that failure to comply fully with those conditions, terms, and requirements or any change in the use of the permit inconsistent with its terms and conditions will be considered a violation and cause for suspension, revocation, or annulment of the permit thereby rendering the permit illegal and subject to appropriate Department action, up to an including removal of the installation at the permittee's expense.

[8] Performance Bond Required? Yes No Company _____
Effective Date _____ Expiration Date _____ Amount \$ _____

[9] This permit shall be void if the work described herein does not comply with the conditions, terms, and requirements applicable to this permit, and if the work is not completed by 08/11/2025

Dated 08/08/2024

Rev 5/6/2021

(the remainder of this page is left blank intentionally)

**General Provisions Applicable to All Permits
(Sections 5515.01 and 5515.02 of O.R.C.)**

- [1] This permit is not a substitute for satisfying the rights or obligations of any other party who may have an interest in the underlying fee interest.
- [2] The granting of this permit does not convey to the permittee or to the property served any rights, title, or interest in state highway rights of way or in the design or operation of the state highway; or in any way abridge the right of the Director of the Department of Transportation in his jurisdiction over state highways. If, in the process of any future work or for the benefit of the traveling public, it becomes necessary, in the opinion of the Director of Transportation to order the removal, reconstruction, relocation, or repair of any of the fixtures, or work performed under this permit, said removal, reconstruction, relocation, or repair shall be wholly at the expense of the owners thereof or the permittee and be made as directed by the Director of Transportation and within the time determined by the Director. Such changes in the state highway design or operation, necessary for improved safety and operation or for the benefit of the traveling public, shall not require a permit modification since the permit confers no private rights to the permittee over the control of the state highway.
- [3] The District Deputy Director acts for and on behalf of the Director in issuing and carrying out the provisions of all permits. The District Deputy Director has full authority to ensure that all provisions of the permit are met and to reject any materials, design, and workmanship that do not meet applicable Department standards. The District Deputy Director, at his/her discretion, may require a performance bond or certified check as a prerequisite to the issuance of a permit.
- [4] Failure on the part of the permittee to comply fully with the provisions and conditions of the permit will be cause for suspension, revocation, or annulment of the permit thereby rendering the permit illegal and subject to appropriate Departmental action. By accepting the permit, the permittee agrees to comply with all conditions, terms, and restrictions printed or written on or attached to the permit. If the permittee or its agent performs any work contrary to the conditions of the permit or to the instructions of the District Deputy Director and, after due notice, fails to correct the problem, the Department of Transportation may, with or without notice, correct or remove such work and the permittee shall reimburse the Department for the costs and shall hold the Department harmless for all results of such work.
- [5] The permittee shall indemnify and hold harmless the State of Ohio, Department of Transportation, its officers, representatives and assigns, from any and all loss, liability, damages, litigation costs, and claims for injury or death to any person, property, or business caused by or resulting from any act, omission, event, consequence, or occurrence, negligent or otherwise of the permittee, its employees, agents, or assigns as a result of the issuance of this permit.
- [6] All work authorized under the permit shall be performed to the Department's satisfaction, and the entire expense shall be borne by the permittee. No work shall be performed until the permittee has contacted the Department's appointed representative named on the permit and received instructions. The Department's representative may inspect all work covered by the permit, or the Department reserves the right, during the time any or all of the work is being performed, to appoint an inspector over the work who shall represent the interest of the State on the work and any compensation arranged for shall be paid wholly by the permit holder. Work not in compliance shall be halted and the District Deputy Director shall be notified of the cause. The permittee shall be notified of the Department's determination and given an opportunity to correct the problem. If the problem is not corrected timely or to the satisfaction of the Department, this permit will be revoked.
- [7] Failure to complete all work within the time specified on the permit shall void the permit, thereby making the permit illegal and subject to appropriate Departmental action. The permittee may request an extension in writing from the District Office, explaining why the extension is necessary and when the work is expected to be completed.
- [8] All work infringing on the pavement or shoulders shall comply with applicable standards and requirements regarding traffic control devices. Failure to comply will be cause for revocation or suspension of the permit. Any closure of lanes or shoulders shall be described in terms of location, duration, time of day, etc. Such work shall not begin until all traffic control devices are in place.

[9] If any grading, sidewalk, or other work allowed by a permit interferes with the drainage of the highway in any way, such catch basins and outlets as necessary shall be constructed to take proper care of said drainage and any materials such as pipes and tiles damaged during any installation or repair by the permittee or its employees or agents shall be repaired immediately at the sole cost of the permittee. Permittee shall timely notify the Department of any such damage and repairs thereto. Failure of the permittee to immediately repair the damage after it is discovered shall result in the Department performing the repair and the permittee shall reimburse the Department for the costs and shall hold the Department harmless for all the results of such work which may include removal of the permittee's facilities.

[10] Any damage to ODOT or another's property caused by the work shall be repaired by the permittee or permittee's agent or contractor in a timely manner and at the sole cost of permittee. If any emergency repairs to ODOT property are needed that cannot be performed by the permittee or permittee's agent or contractor, ODOT shall cause the repairs to be performed at the sole cost of permittee.

[11] Upon completion of the work, the permittee shall leave the highway clean of all rubbish, excess materials, temporary structures and equipment, and all parts of the highway shall be left in a condition acceptable to the Department. Upon satisfactory completion of the work authorized by the permit, the Department's appointed representative shall complete the Permit Inspection Certificate, Form No. MR 678 certifying that the permittee has complied with the terms of the permit.

[12] Except as herein authorized, no excavation shall be made or obstacle placed within the limits of the highway so as to interfere with the travel over the road.

[13] All pole lines are to be built in accordance with Rule 4901:3-1-08 of Ohio Administrative Code promulgated and enforced by the Public Utilities Commission of Ohio.

[14] All underground utilities shall be installed at a depth and horizontal distance from the road surface and any appurtenances in accordance with state and national safety standards and as pre-approved by the Department. After installation, the exact location of the utility shall be provided to the Department. The Department shall be held harmless for any damage to utilities due to insufficient or inaccurate installation or identification and all repairs shall be at the sole cost of the permittee.

[15] The permittee shall comply with the Air Pollution requirements of Rule 3745-17-08 of the Ohio Administrative Code promulgated and enforced by the Ohio Environmental Protection Agency.

[16] The permittee certifies that he or she is fully authorized to sign this permit. This permit shall apply to and be binding upon the permittee and any successors in interest. No change in ownership of the underlying property or of the facility owned by permittee shall in any way alter the permittee's obligations under this permit.

[17] The permittee(s) for herself/himself/themselves/itself, her/his/their/its personal representatives, and her/his/their/its successors in interest and assigns, as a part of the consideration hereof, do/does hereby covenant and agree that:

(1) No person on the grounds of race, color, or national origin, shall be excluded from participation in, be denied the benefits of, or be otherwise subjected to discrimination in the use of the utility/facilities/ services of the permittee.

(2) In the construction of any improvements on, over, or under the above described property and the furnishing of services thereon, no person on the grounds of race, color, national origin, sex, age, or disability shall be excluded from the participation in, be denied the benefits of, or be otherwise subjected to discrimination.

(3) The above described property shall be used in a manner that at all times is in compliance with all other requirements imposed by or pursuant to Title 49, Code of Federal Regulations, U.S. DOT, Subtitle A, Office of the Secretary, Part 21, Non-discrimination in Federally-assisted programs of the U.S. DOT — Effectuation of Title VI of the Civil Rights Act of 1964, and as said Regulations may be amended.

(4) In the event that this instrument grants a lease, license, or permit and any of the above non-discrimination covenants is breached, then the State of Ohio, Department of Transportation, shall have the unfettered right to terminate the lease, license or permit and to re-enter and repossess the above-described property and hold the same as if said lease, license or permit had never been made or issued.

This permit is granted subject to the following attached conditions:

Permittee shall widen State Route 149 to accommodate a right and left turn lane and construct a commercial drive for access on behalf of Love's Travel Stops & Country Stores, Inc on the West side of State Route 149 in Belmont County. The drive is approximately 645 feet South of Reco Drive. The work shall include the signals being upgraded at both of the ramps of I70 and SR 149, In addition temporary wood pole signals will be installed at the intersection with Reco Drive. The permittee is to make minor widening improvements to Reco Drive. The drive opening shall be constructed as per ODOT standard roadway drawing BP-4.1. The apron profile & pavement build up shall be constructed as per ODOT L&D Manual, volume 1, section 803.1, section 803.2 and section 805.1 & 805.2 and Figure 401-2E. All work shall comply with State & Federal guidelines & in no way should the work adversely affect the travelling public. "See Additional Requirements."

This permit is valid only within the limits of right-of-way of this state route. Permits for that portion of your facilities located along county or township right-of-way must be obtained from the appropriate authorities. **A copy of your permit is to be on-site at all times while working.**

All work required for this improvement shall be subject to inspection by the State of Ohio or their designated representative.

All work to be performed at no cost to the State or Federal Highway Administration.

The permittee and the State of Ohio Department of Transportation are to enter into an agreement for the completion of this work.

The Permittee agrees that the State of Ohio, Department of Transportation, and Federal Highway Administration shall be saved harmless from any and all claims or damages, public or private, arising from or growing out of the issuance of this permit.

Highway Lighting and/or traffic signals: even though ODOT is listed as a member of the Ohio utilities protection service (OUPS), the contractor is required to contact ODOT directly so that the ODOT utilities located within this project are marked. The contractor shall notify ODOT project engineer/project supervisor, Jacob Petry at 330-204-6780, fourteen (14) calendar days in advance of any work, for the need to mark ODOT owned utilities.

ODOT's District 11 Public Information Officer (PIO) must be notified 72 hours prior to any work for the purpose of issuing a press release through the local media. Please fill out the form on D11 Permit PIO Notification - Formstack https://odot.formstack.com/forms/d11_permit_pio_notification

The Permittee is responsible for maintaining the integrity of the Edge of Pavement at all times during and after work is complete. If there is any damage to this State Route it will be the permittees responsibility to repair.

The updated maintenance of traffic sheets dated 8/8/24 shall govern.

C-01 To assure the proper installation, the Ohio Department of Transportation County Manager or his representative must be notified a minimum of 24 hours prior to any work being started and must be present to approve grades, location and material used. All cost associated with having a construction inspector on a site will be paid by the permittee.

C-02 There will be no parking, loading/unloading of equipment, service vehicles, erection of lights or placing advertising devices within the state highway right-of-way. Similarly, no equipment, service vehicles, devices or structures are permitted to overhang the state highway.

- C-03 The Advertising Device Control Section in Columbus, Ohio must be notified and must give their authorization prior to erecting any advertising device. All requests should be mailed to the Ohio Department of Transportation Advertising Device Control, 1980 W. Broad St., P.O. Box 899, Columbus, Ohio, 43223. All on premise devices must be in accordance with Section 5501:2-2-02 part (C) of the Ohio Administrative Code. An on-premise device visible to an interstate system must additionally conform to Section 5501:2-2-03 Part (A) of the Ohio Administrative Code.
- C-04 The drainage and/or driveway pipe if required shall be supplied by the Permittee, at the Permittee's cost, in the size and to the specifications indicated by the Ohio Department of Transportation, and installed subject to inspection and approval by the Ohio Department of Transportation.
- C-05 The approach will slope down and away from the through pavement edge at a minimum of 1.6% as per Location and Design Manual Vol. 1, Section 400, Figure 401-2E. No surface water will be allowed to drain onto the highway pavement.
- C-06 The drive is to be paved at least 50' back from the Edge of Pavement, see Location and Design Manual Vol. 1 Section 805.3.
- C-07 Permittee to furnish all labor, material and equipment necessary to complete and maintain the project.
- C-08 The contractor must give property owners a minimum of 24-hour notice before cutting any driveways. Also, the permit holder shall give ample notice to all property owners before trenching or cable plowing operations begin.
- C-09 The use of traveled lanes stabilized or un-stabilized berms for the storage of construction materials or depositing of excavated material is NOT permitted. All excess and discarded material to be removed from State Right of Way.
- C-10 The portion of the earthwork on State right of way shall be seeded and mulched to prevent erosion, as per Item 659 of the Ohio Department of Transportation Construction and Material Specifications Manual.
- C-11 Private drainage outlets, such as field tiles, disturbed by this operation shall be restored to working order to the satisfaction of the owner.
- C-12 The permittee shall take any and all appropriate measures to limit soil erosion during and after construction authorized herein. As such, he shall be fully accountable to the Ohio EPA, the Soil Conservation Service and other appropriate agencies for any violation or disregard of the applicable governing standards and regulations related to the protection and conservation of soils that are affected by this permitted work - Ref. SS877.
- C-13 All public and private property, including highway fence, that is disturbed by the contractor will be repaired to a condition equal to or better than the original condition, including sidewalks and driveways.
- C-14 This work will be performed at no cost to the State or the Federal Highway Administration.
- C-15 Any mud, snow or debris that accumulates on the highway as a result of this project (i.e., from tire tracks, equipment, etc.) is to be removed immediately at the Permittee's expense.
- C-16 The Permittee is responsible for complying with any/all applicable state and/or federal environmental laws including, but not limited to, obtaining any necessary Section 404 & 401 waterway permits prior to performing any work within the state right-of-way.
- C-17 Backfill is to be compacted to ODOT specifications.
- C-18 All work on State Right of Way to be performed in a manner satisfactory to the Ohio Department of Transportation.
- C-19 The Permittee agrees that the State of Ohio, Department of Transportation, and Federal Highway Administration shall be saved harmless from any and all claims or damages, public or private, arising from or growing out of the issuance of this permit.

C-20 Work is not to be performed during inclement weather conditions (i.e., ice, snow, fog, heavy rainstorms, etc.). Additionally, work is not to start until one (1) hour after sunrise and is to cease one (1) hour before sunset.

C-21 It is to be understood that should the location and/or design (construction) of the new access and turn lanes interfere with the safety of the traveling public, or drainage issues develop undermining the highway's foundation or cause neighboring properties to flood, ODOT District 11 reserves the right to require modification and/or realignment of these newly installed facilities.

C-22 All work requiring men or vehicles on the pavement or shoulders shall comply fully with the Ohio Manual of Uniform Traffic Control Devices (see highlighted link below) for Construction and Maintenance Operations and Item 614 (Maintaining Traffic) in the State of Ohio Department of Transportation Construction and Material Specifications Manual. Failure to comply with this requirement will be just cause for immediate suspension of this permit until such time the proper traffic control is in place.

http://www.dot.state.oh.us/Divisions/Engineering/Roadway/DesignStandards/traffic/OhioMUTCD/Pages/OMUTCD2012_current_default.aspx

C-23 Traffic to be maintained as per the Ohio Manual of Uniform Traffic Control Devices. All traffic to be maintained by above requirements and applicable sections of the OMUTCD and Item 614 of the Construction and Material Specifications Manual.

C-24 All areas where the vegetative growth has been disturbed or destroyed by the contractor/utility company shall be fertilized, seeded and mulched as per the 2019 ODOT Construction and Material Specifications Item 659 Seeding and Mulching. The contractor shall maintain the Temporary Sediment and Erosion Control as per the ODOT Construction and Material Specifications. All berm, shoulder or roadside slopes disturbed or destroyed shall be restored to the original or better conditions.

C-25 All excess and discarded material to be removed from the State Right of Way.

C-26 The Permittee is held responsible for all public and private utility coordination and relocation required in the performance of the work

C-27 NOTICE: Failure to notify the Ohio Department of Transportation County Manager or his representative a minimum of 24 hours prior to any work being started and again upon completion of the project will void this permit.

Failure to comply with these terms will result in the revocation of the permit and subsequent removal of the driveway.

IF, WITHIN SEVEN (7) DAYS OF COMPLETION OF WORK AND INSPECTION OF THE PROJECT BY AN O.D.O.T. REPRESENTATIVE, THE PERMIT REQUIREMENTS HAVE NOT BEEN FULFILLED BY THE PERMITTEE, O.D.O.T. MAY REMOVE OR REPAIR THE WORK AT THE PROPERTY OWNER'S EXPENSE AND NO FUTURE PERMITS FOR THIS TYPE OF WORK WILL BE ISSUED TO YOU.

THE FUTURE MAINTENANCE OF ANY WORK PERMITTED HEREIN SHALL BE THE RESPONSIBILITY AND AT THE EXPENSE OF THE PERMITTEE.

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STATE OF OHIO
DEPARTMENT OF TRANSPORTATION

BOND

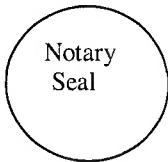
Know all persons by these presents, that we, Love's Travel Stops & Country Stores, Inc. of 10601 N. Pennsylvania Ave., Oklahoma City, OK 73120 as Principal, and Fidelity and Deposit Company of Maryland as Surety, are held and firmly bound unto the Ohio Department of Transportation in the sum of One Million Five Hundred Ninety Five Thousand and No/100 Dollars (\$ 1,595,000.00) lawful money of the United States, for which payment will and truly to be made, we bind ourselves, our heirs, executors, administrators, successors and assigns, jointly and severally, firmly by these presents.

Whereas, the above bound Principal has been granted or is about to be granted a permit for the below listed work within the Ohio Department of Transportation Right of Way, to wit:
St. Clairsville, Ohio Roadway and Signal Plan Work.

Now, therefore, the Conditions of this Obligation are such that in addition to complying with all applicable laws, rules, regulations, policies and ordinances related to the issuance and coverage of the permit, the above Principal shall indemnify and hold harmless the Ohio Department of Transportation against all loss, costs, expense or damage caused by Principal resulting from the issuance of the above described permit.

The term of this bond is for a period beginning on the 17th day of June, 2024 and ending* on the 21st day of January, 2025.

Signed, sealed and dated this 21st day of June 2024



Principal, Love's Travel Stops & Country Stores, Inc.

By [Signature]

Surety, Fidelity and Deposit Company of Maryland

By [Signature]

Tonie Petranek, Attorney-in-Fact

***Notice to the Permittee and Bonding Agencies:** The initial time frame of this bond is a projection to when all the work within the State's right-of-way is to be completed, final inspection completed and work approved. Should the permittee fail to complete all work within the projected time frame, the permittee shall be responsible to insure this bond remains active. This bond shall be released only by formal written notice by the Ohio Department of Transportation.

**ZURICH AMERICAN INSURANCE COMPANY
COLONIAL AMERICAN CASUALTY AND SURETY COMPANY
FIDELITY AND DEPOSIT COMPANY OF MARYLAND
POWER OF ATTORNEY**

KNOW ALL MEN BY THESE PRESENTS: That the ZURICH AMERICAN INSURANCE COMPANY, a corporation of the State of New York, the COLONIAL AMERICAN CASUALTY AND SURETY COMPANY, a corporation of the State of Illinois, and the FIDELITY AND DEPOSIT COMPANY OF MARYLAND a corporation of the State of Illinois (herein collectively called the "Companies"), by Robert D. Murray, Vice President, in pursuance of authority granted by Article V, Section 8, of the By-Laws of said Companies, which are set forth on the reverse side hereof and are hereby certified to be in full force and effect on the date hereof, do hereby nominate, constitute, and appoint Ricardo J. REYNA, Tina MCEWAN, Don E. CORNELL, Joshua SAUNDERS, Robbi MORALES, Sophinie HUNTER, Kelly A. WESTBROOK, Tonie PETRANEK, Mikaela PEPPERS of Dallas, Texas, its true and lawful agent and Attorney-in-Fact, to make, execute, seal and deliver, for, and on its behalf as surety, and as its act and deed: **any and all bonds and undertakings**, and the execution of such bonds or undertakings in pursuance of these presents, shall be as binding upon said Companies, as fully and amply, to all intents and purposes, as if they had been duly executed and acknowledged by the regularly elected officers of the ZURICH AMERICAN INSURANCE COMPANY at its office in New York, New York., the regularly elected officers of the COLONIAL AMERICAN CASUALTY AND SURETY COMPANY at its office in Owings Mills, Maryland., and the regularly elected officers of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND at its office in Owings Mills, Maryland., in their own proper persons.

The said Vice President does hereby certify that the extract set forth on the reverse side hereof is a true copy of Article V, Section 8, of the By-Laws of said Companies, and is now in force.

IN WITNESS WHEREOF, the said Vice-President has hereunto subscribed his/her names and affixed the Corporate Seals of the said **ZURICH AMERICAN INSURANCE COMPANY, COLONIAL AMERICAN CASUALTY AND SURETY COMPANY, and FIDELITY AND DEPOSIT COMPANY OF MARYLAND**, this 14th day of September, A.D. 2023.



ATTEST:
**ZURICH AMERICAN INSURANCE COMPANY
COLONIAL AMERICAN CASUALTY AND SURETY COMPANY
FIDELITY AND DEPOSIT COMPANY OF MARYLAND**

By: *Robert D. Murray*
Vice President

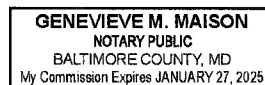
By: *Dawn E. Brown*
Secretary

**State of Maryland
County of Baltimore**

On this 14th day of September, A.D. 2023, before the subscriber, a Notary Public of the State of Maryland, duly commissioned and qualified, **Robert D. Murray, Vice President and Dawn E. Brown, Secretary** of the Companies, to me personally known to be the individuals and officers described in and who executed the preceding instrument, and acknowledged the execution of same, and being by me duly sworn, depose and saith, that he/she is the said officer of the Company aforesaid, and that the seals affixed to the preceding instrument are the Corporate Seals of said Companies, and that the said Corporate Seals and the signature as such officer were duly affixed and subscribed to the said instrument by the authority and direction of the said Corporations.

IN TESTIMONY WHEREOF, I have hereunto set my hand and affixed my Official Seal the day and year first above written.

Genevieve M. Maison



Authenticity of this bond can be confirmed at bondvalidator.zurichna.com or 410-559-8790

EXTRACT FROM BY-LAWS OF THE COMPANIES

"Article V, Section 8. Attorneys-in-Fact. The Chief Executive Officer, the President, or any Executive Vice President or Vice President may, by written instrument under the attested corporate seal, appoint attorneys-in-fact with authority to execute bonds, policies, recognizances, stipulations, undertakings, or other like instruments on behalf of the Company, and may authorize any officer or any such attorney-in-fact to affix the corporate seal thereto; and may with or without cause modify or revoke any such appointment or authority at any time."

CERTIFICATE

I, the undersigned, Vice President of the ZURICH AMERICAN INSURANCE COMPANY, the COLONIAL AMERICAN CASUALTY AND SURETY COMPANY, and the FIDELITY AND DEPOSIT COMPANY OF MARYLAND, do hereby certify that the foregoing Power of Attorney is still in full force and effect on the date of this certificate; and I do further certify that Article V, Section 8, of the By-Laws of the Companies is still in force.

This Power of Attorney and Certificate may be signed by facsimile under and by authority of the following resolution of the Board of Directors of the ZURICH AMERICAN INSURANCE COMPANY at a meeting duly called and held on the 15th day of December 1998.

RESOLVED: "That the signature of the President or a Vice President and the attesting signature of a Secretary or an Assistant Secretary and the Seal of the Company may be affixed by facsimile on any Power of Attorney...Any such Power or any certificate thereof bearing such facsimile signature and seal shall be valid and binding on the Company."

This Power of Attorney and Certificate may be signed by facsimile under and by authority of the following resolution of the Board of Directors of the COLONIAL AMERICAN CASUALTY AND SURETY COMPANY at a meeting duly called and held on the 5th day of May, 1994, and the following resolution of the Board of Directors of the FIDELITY AND DEPOSIT COMPANY OF MARYLAND at a meeting duly called and held on the 10th day of May, 1990.

RESOLVED: "That the facsimile or mechanically reproduced seal of the company and facsimile or mechanically reproduced signature of any Vice-President, Secretary, or Assistant Secretary of the Company, whether made heretofore or hereafter, wherever appearing upon a certified copy of any power of attorney issued by the Company, shall be valid and binding upon the Company with the same force and effect as though manually affixed.

IN TESTIMONY WHEREOF, I have hereunto subscribed my name and affixed the corporate seals of the said Companies, this 21st day of June, 2024.



Thomas O. McClellan

Thomas O. McClellan
Vice President

TO REPORT A CLAIM WITH REGARD TO A SURETY BOND, PLEASE SUBMIT A COMPLETE DESCRIPTION OF THE CLAIM INCLUDING THE PRINCIPAL ON THE BOND, THE BOND NUMBER, AND YOUR CONTACT INFORMATION TO:

Zurich Surety Claims
1299 Zurich Way
Schaumburg, IL 60196-1056
reportsfclaims@zurichna.com
800-626-4577

Authenticity of this bond can be confirmed at bondvalidator.zurichna.com or 410-559-8790

Parcel Number 39-00338.000	Legal Description R5 T8 S14 HWY EASE .09 67.9601 AC NEW SURVEY	Location 66320 BELMONT MORRISTOWN
Owner LOVE'S TRAVEL STOPS & COUNTRY STORES INC.		Acres 67.9601

Property	
Tax District 39-UNION TWP - UNION LSD	School District UNION LSD
Neighborhood 20000-UNION LOCAL & HARRISON LOCAL SCHOOL DIST	Subdivision
Map Number 14.00	Routing Number 39-01400-007000
CD Year	

Owner
Contact LOVE'S TRAVEL STOPS & COUNTRY STORES INC. PO BOX 26210 OKLAHOMA CITY OK 73126-0210
Owner Name LOVE'S TRAVEL STOPS & COUNTRY STORES INC.

Deed		
Legal Description R5 T8 S14 HWY EASE .09 67.9601 AC NEW SURVEY	Acres 67.9601	
Date Sold 05/23/2023	Sales Amount 3,450,000.00	Volume / Page 912 / 944

Taxpayer
Contact LOVE'S TRAVEL STOPS & COUNTRY STORES INC. PO BOX 26210 OKLAHOMA CITY OK 73126-0210

Values
Land Use 499-OTHER COMMERCIAL STRUCTURES

Current Charges		
Full Rate 52.950000	Effective Rate 39.451213	Qualifying Rate 36.390872

	Appraised			
Land	298,610			
Improvement	481,760			
Total	780,370			
CAUV (N)	0			
Homestead (N)	0			
Owner Occupancy (N)	0			
Taxable	780,370			
		Prior	First	Second
		Tax	0.00	5,387.70
		Special	0.00	2.06
		Total	0.00	5,389.76
		Paid	0.00	5,389.76
		Due	0.00	0.00
Adding on? Try our tax estimator.				

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Future Charges

Type	Description	A
Special Assessments	C9700 JEFFERSON BELMONT SOLID WASTE	2

Loves - St. Clairsville, OH - Access Vicinity Map

Legend
📌 Access Drive Location

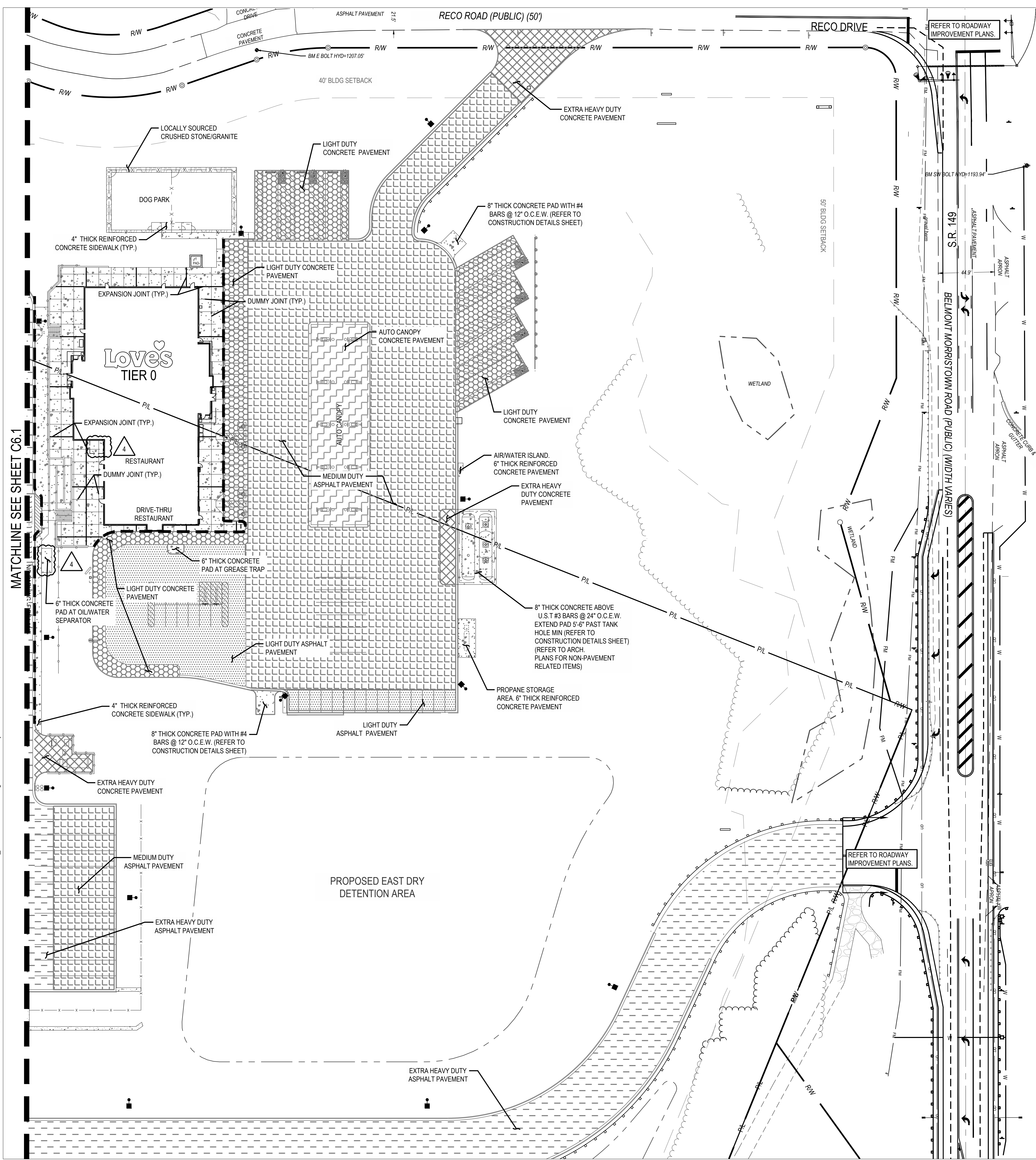


Proposed Love's Access Drive (utilizing existing drive apron)

LOVES - ST. CLAIRSVILLE 📌

1000 ft

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\DOT-COMBINED\759267_SITE PAVEMENT.dwg - 4/12/2024 - Ryan Ash



LEGEND

EXISTING
REFER TO ALTA/SPS TOPOGRAPHIC SURVEY

PROPOSED

	PROPOSED EXTRA HEAVY DUTY ASPHALT PAVEMENT		PROPOSED EXTRA HEAVY DUTY CONCRETE PAVEMENT
	PROPOSED HEAVY DUTY ASPHALT PAVEMENT		PROPOSED HEAVY DUTY CONCRETE PAVEMENT
	PROPOSED MEDIUM DUTY ASPHALT PAVEMENT		PROPOSED MEDIUM DUTY CONCRETE PAVEMENT
	PROPOSED LIGHT DUTY ASPHALT PAVEMENT		PROPOSED LIGHT DUTY CONCRETE PAVEMENT
	GRAVEL		PROPOSED SIDEWALK/ CONCRETE PAVEMENT
			PROPOSED AUTO CANOPY CONCRETE PAVEMENT
			PROPOSED TRUCK CANOPY CONCRETE PAVEMENT

--- COLORED CONCRETE CURB

NOTE: REFER TO CONSTRUCTION DETAILS

- ### PAVING NOTES
- PAVEMENT SECTION DESIGNS ARE TAKEN FROM THE GEOTECHNICAL INVESTIGATION REPORT PREPARED BY TERRACON DATED JULY 21, 2023. REFERENCE THIS REPORT FOR COMPLETE DESIGN SPECIFICATIONS.
 - WITHIN 24 HOURS PRIOR TO ANY PAVING OPERATION, THE CONTRACTOR SHALL PERFORM SUBGRADE COMPACTION AND PROOF ROLL TESTS ON PREPARED SUBGRADE TO VERIFY SUBGRADE HAS NOT DEGRADATED DURING PROJECT CONSTRUCTION.
 - ALL CONCRETE SHALL HAVE LIGHT BROOM FINISH EXCEPT TRUCK FUELING BAYS WHICH SHALL HAVE HEAVY BROOM FINISH.
 - THE FOLLOWING AREAS ARE TO BE SEALED 30 DAYS AFTER CONCRETE IS POURED. REFERENCE ARCHITECTURAL SPECIFICATIONS:
 - ALL SIDEWALKS
 - AUTO CANOPY PAD
 - TRUCK CANOPY PAD
 - FUEL DISPENSER ISLANDS AND BUMPERS
 - BUILDING PARKING SPACES AT FRONT OF BUILDING
 - EXTERIOR CONCRETE AT TIRE SHOP
 - TIRE SHOP PAD
 - DUMPSTER PAD AREA
 - OIL-WATER SEPARATOR PAD
 - U.S.T. SLAB
 - DRIVE THRU LANE
 - ALL ASPHALT THAT ABUTS CONCRETE IS TO BE FINISHED AT 1/2 INCH ABOVE FINISHED CONCRETE ELEVATION.
 - ALL PAVING SHALL CONFORM TO ODOT STANDARDS AND SPECIFICATIONS.
 - AFTER COMPLETING THE PAVING OPERATION THE CONTRACTOR IS RESPONSIBLE FOR HAVING THE PAVEMENT CORED UNDER THE SUPERVISION OF A MATERIALS INSPECTOR AND SUBMITTING CORE RESULTS TO ENGINEER AND OWNER. FOR ASPHALT PAVEMENT, CONTRACTOR SHALL CORE BASE AND INTERMEDIATE COURSES ONLY. SURFACE COURSE SHOULD NOT BE CORED.
 - REFERENCE GRADING AND DRAINAGE SHEETS FOR PAVEMENT ELEVATION DETAILS.
 - CONCRETE-TO-ASPHALT STANDARD PAVEMENT HEADER TO BE USED FOR ALL CONCRETE-TO-ASPHALT INTERFACES.
 - ASPHALT SHALL NOT ABUT TOP OF SCALE EXCEPT AT ENTRY-EXIT SIDES.
 - PROVIDE 1/2" ISOLATION JOINT WITH SEALED TOP AT PERIMETER OF FUEL ISLANDS, ALL CANOPY COLUMNS AND BOLLARDS. TYPICAL FOR ALL FUELING ISLANDS. REFER TO SHEET C9.0.
 - REFER TO ARCHITECTURAL PLANS FOR CANOPY JOINTING PLAN FOR BOTH AUTO AND DIESEL CANOPIES.



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LOVE'S TRAVEL STOP

ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
ID	Description	Date
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
Scale: 1" = 40'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

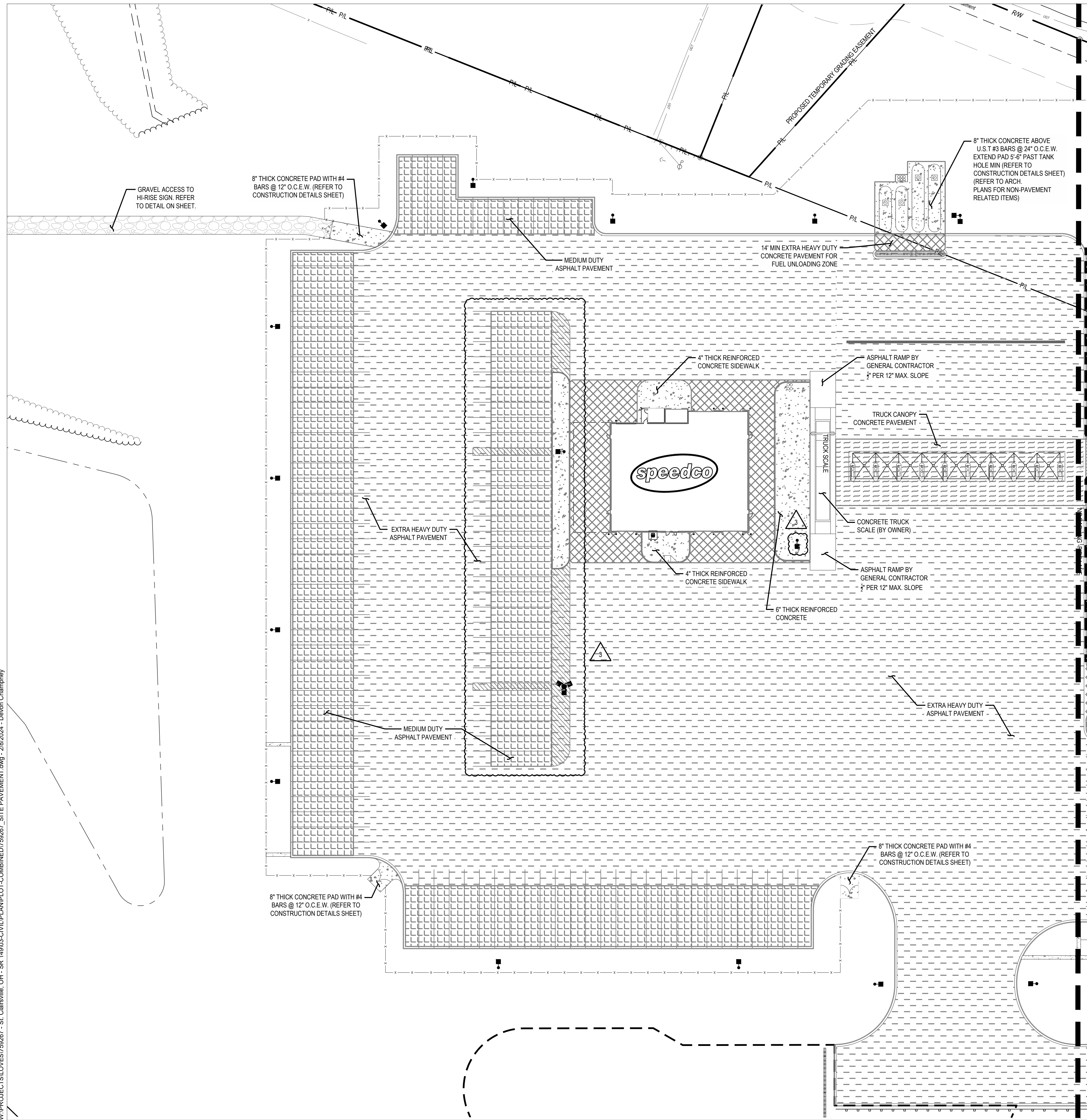
Drawing Title:
PAVEMENT PLAN - EAST

C6.0



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\DOT-COMBINED\759267 SITE PAVEMENT.dwg - 2/19/2024 - Devon Champney



MATCHLINE SEE SHEET C6.0

LEGEND

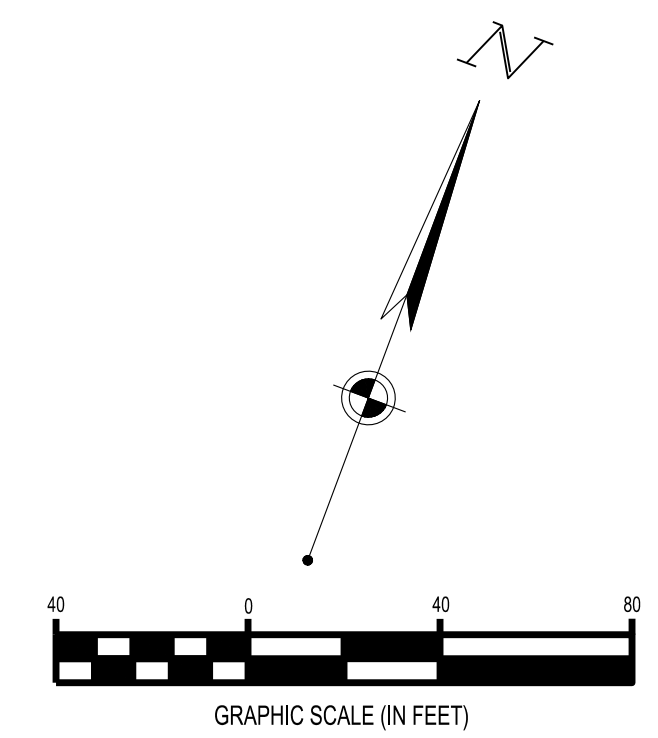
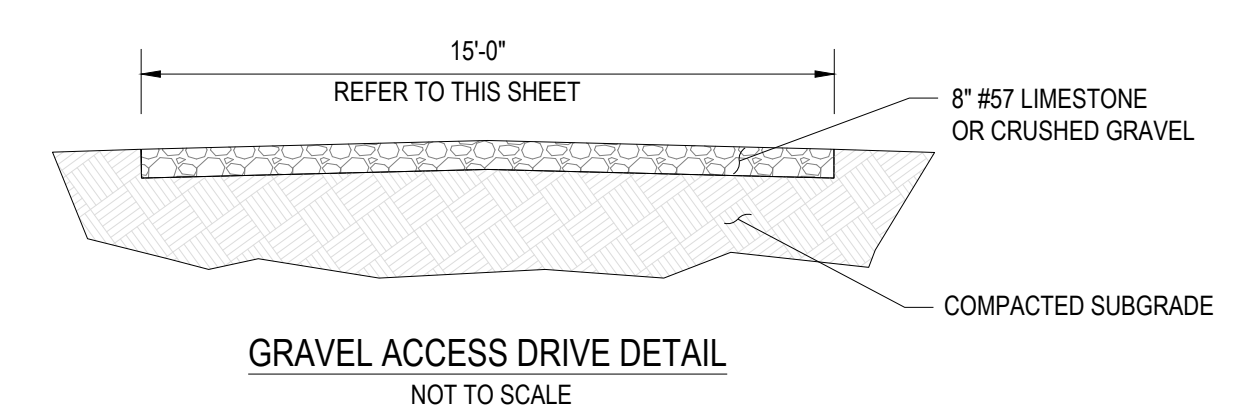
REFER TO ALTANSPS TOPOGRAPHIC SURVEY

EXISTING	PROPOSED
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PROPOSED HEAVY DUTY ASPHALT PAVEMENT	PROPOSED HEAVY DUTY CONCRETE PAVEMENT
PROPOSED MEDIUM DUTY ASPHALT PAVEMENT	PROPOSED MEDIUM DUTY CONCRETE PAVEMENT
PROPOSED LIGHT DUTY ASPHALT PAVEMENT	PROPOSED LIGHT DUTY CONCRETE PAVEMENT
GRAVEL	PROPOSED SIDEWALK/ CONCRETE PAVEMENT
	PROPOSED AUTO CANOPY CONCRETE PAVEMENT
	PROPOSED TRUCK CANOPY CONCRETE PAVEMENT
	COLORLED CONCRETE CURB

NOTE: REFER TO CONSTRUCTION DETAILS

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LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

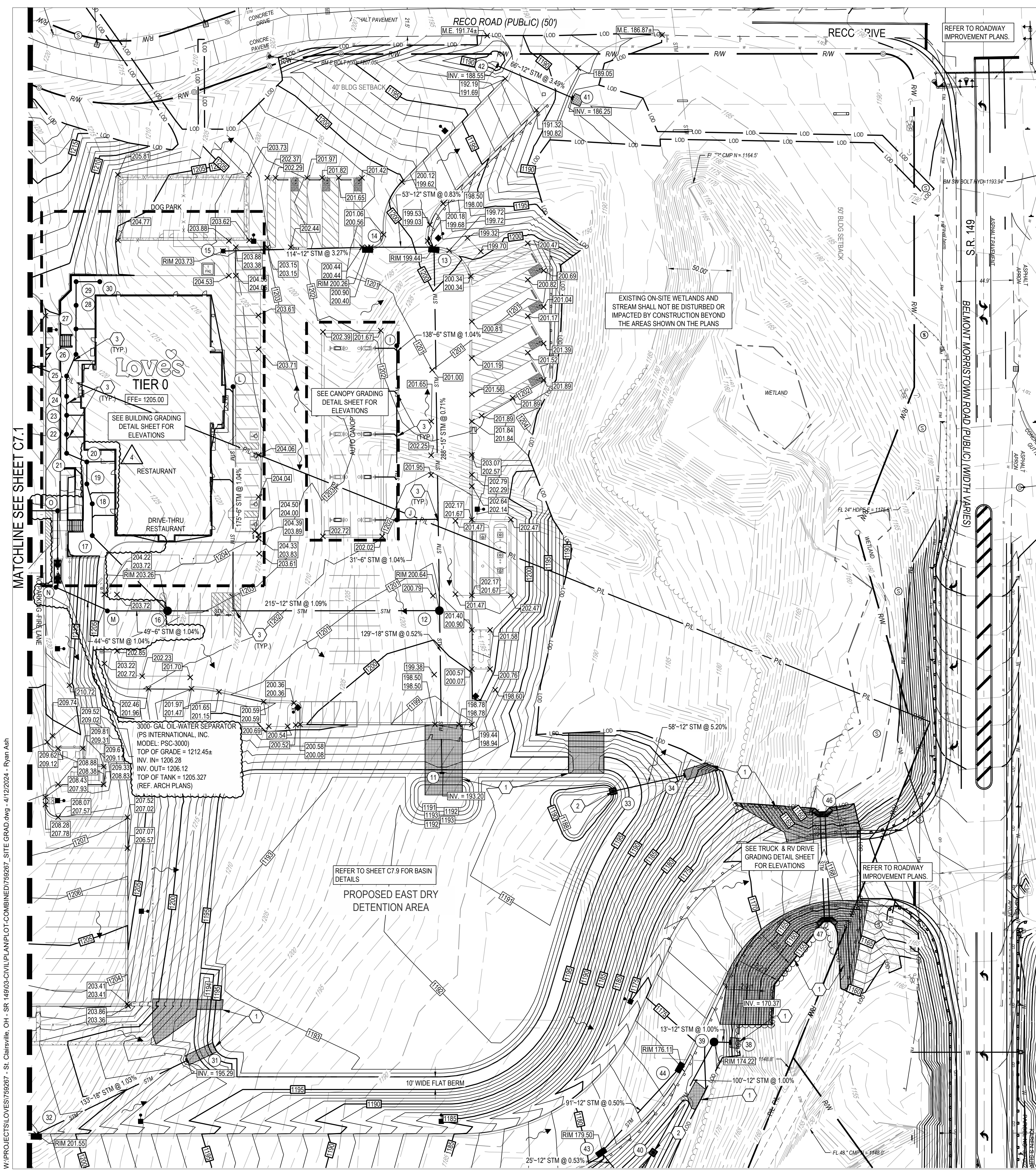
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ID	Description	Date
3	ADDENDUM 3	02/09/2024

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Project Number: 759267
Scale: 1" = 40'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
PAVEMENT PLAN - WEST

C6.1



LEGEND

EXISTING
REFER TO ALTA/SPS TOPOGRAPHIC SURVEY

PROPOSED

1080	MAJOR CONTOUR
075	MINOR CONTOUR
---	RIDGE LINE
---	STORM SEWER
---	STORM SEWER UNDERDRAIN
---	STORM CURB INLET PER CONSTRUCTION DETAILS
---	STORM HALF-HEIGHT HEADWALL PER ODOT HW-2.1 DETAIL

●	STORM CLEANOUT
●	YARD DRAIN
●	SANITARY STANDARD MANHOLE PER CONSTRUCTION DETAILS
●	SANITARY CLEANOUT
●	SPOT ELEVATION
XXXXXX	TOP OF CURB
XXXXXX	BOTTOM OF CURB
---	RIM
---	TOP OF CASTING ELEVATION
---	RIP RAP
---	FLOW ARROW

STORM SEWER STRUCTURE SCHEDULE

NO.	STRUCTURE	RIM	INVERT
11	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1193.20	(18') N
12	48" STORM MANHOLE	1200.64	1194.12 (15') N 1194.37 (12') W 1193.87 (18') S
13	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1199.44	1196.44 (12') W 1196.15 (15') S
14	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1200.26	1196.88 (12') W 1196.88 (12') E
15	NYLOPAST 15" INLINE DRAIN	1203.72	1200.62 (12') E
16	48" STORM MANHOLE	1203.26	1196.72 (12') NW 1205.11 (8') W 1196.72 (12') E
17	NYLOPAST 15" INLINE DRAIN	1204.62	1200.40 (12') N 1200.40 (12') SE
18	NYLOPAST 15" INLINE DRAIN	1204.73	1200.52 (12') NW 1200.52 (12') S
19	NYLOPAST 15" INLINE DRAIN	1204.81	1200.60 (12') N 1200.60 (12') SE
20	NYLOPAST 15" INLINE DRAIN	1204.81	1200.69 (12') W 1200.69 (12') S
21	NYLOPAST 15" INLINE DRAIN	1204.78	1200.78 (12') N 1200.78 (12') E
22	NYLOPAST 15" INLINE DRAIN	1204.78	1200.85 (12') N 1200.85 (12') S
23	NYLOPAST 15" INLINE DRAIN	1204.78	1200.93 (12') N 1200.93 (12') S
24	NYLOPAST 15" INLINE DRAIN	1204.84	1201.17 (8') N 1201.00 (12') S
25	NYLOPAST 10" INLINE DRAIN	1204.81	1201.34 (8') N 1201.34 (8') S
26	NYLOPAST 10" INLINE DRAIN	1204.77	1201.53 (8') N 1201.53 (8') S
27	NYLOPAST 10" INLINE DRAIN	1204.77	1201.74 (8') N 1201.74 (8') S
28	NYLOPAST 10" INLINE DRAIN	1204.77	1202.02 (6') N 1201.94 (8') S
29	NYLOPAST 10" INLINE DRAIN	1204.79	1202.22 (6') E 1202.22 (6') S
30	NYLOPAST 10" INLINE DRAIN	1205.76	1202.41 (6') W
31	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1197.33	1195.29 (18') SW
32	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1201.55	1196.66 (18') NE
33	OUTLET CONTROL STRUCTURE	1192.50	1190.00 (12') NE
34	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1188.55	1187.00 (12') SW
38	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1171.92	1170.37 (12') W
39	48" STORM MANHOLE	1174.21	1170.50 (12') S 1170.50 (12') E
40	OUTLET CONTROL STRUCTURE	1174.00	1171.50 (12') N
41	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1187.80	1186.25 (12') W
42	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1190.10	1188.55 (12') E
42	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1174.00	1172.45 (12') NW
43	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1174.00	1172.45 (12') NW
43	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1179.54	1172.58 (12') N 1172.58 (12') SE
44	48" INLET MANHOLE	1179.54	1172.58 (12') N 1172.58 (12') SE
44	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1176.04	1173.04 (12') S
45	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1176.04	1173.04 (12') S
46	ODOT FULL-HEIGHT HEADWALL PER STD. DWG. HW-1.1	1158.92	1154.00 (48') SE
47	ODOT FULL-HEIGHT HEADWALL PER STD. DWG. HW-1.1	1157.52	1152.60 (48') NW

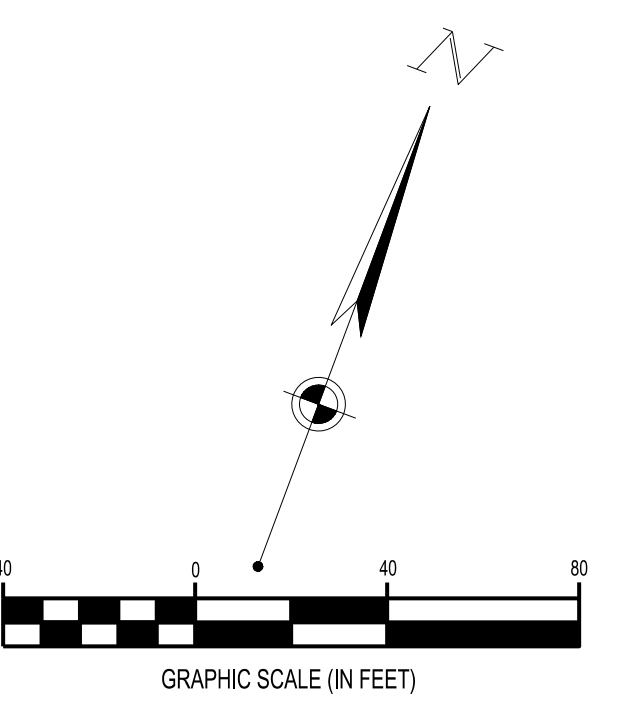
STORM SEWER CLEANOUT SCHEDULE

NO.	STRUCTURE	RIM	INVERT
I	6" CLEANOUT	1201.50	1198.50 (6') S
J	6" CLEANOUT	1201.88	1197.07 (6') N 1197.07 (6') E
L	6" CLEANOUT	1204.80	1197.95 (6') S
M	6" CLEANOUT	1207.51	1205.62 (6') W 1205.62 (6') E
N	6" CLEANOUT	1212.21	1206.08 (6') N 1206.08 (6') E
O	6" CLEANOUT	1212.68	1206.69 (6') W 1206.69 (6') S

REFER TO CONSTRUCTION DETAILS SHEET
REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
REFER TO SHEETS C7.13 TO C7.15 FOR STORM SEWER PROFILES

- CODED NOTES**
- CONTRACTOR TO INSTALL EROSION CONTROL FLEXAMAT STANDARD OR APPROVED EQUAL.
 - OUTLET STRUCTURE. REFER TO SHEETS C7.9 TO C7.12.
 - WYE CONNECTION
 - (2) 1-1/2" CONDUITS FOR 4" SUMP. COORDINATE WITH CAT SCALE REPRESENTATIVE AND GENERAL CONTRACTOR.

A DETAILED GEOTECHNICAL REPORT HAS BEEN PREPARED FOR THIS SITE AND SHOULD BE ADHERED TO THE FULL EXTENT OF THOSE RECOMMENDATIONS AND FIELD TESTING. REFER TO GEOTECHNICAL ENGINEERING REPORT PREPARED BY TERRACON PROJECT NO.422529



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



LOVES TRAVEL STOP
ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions

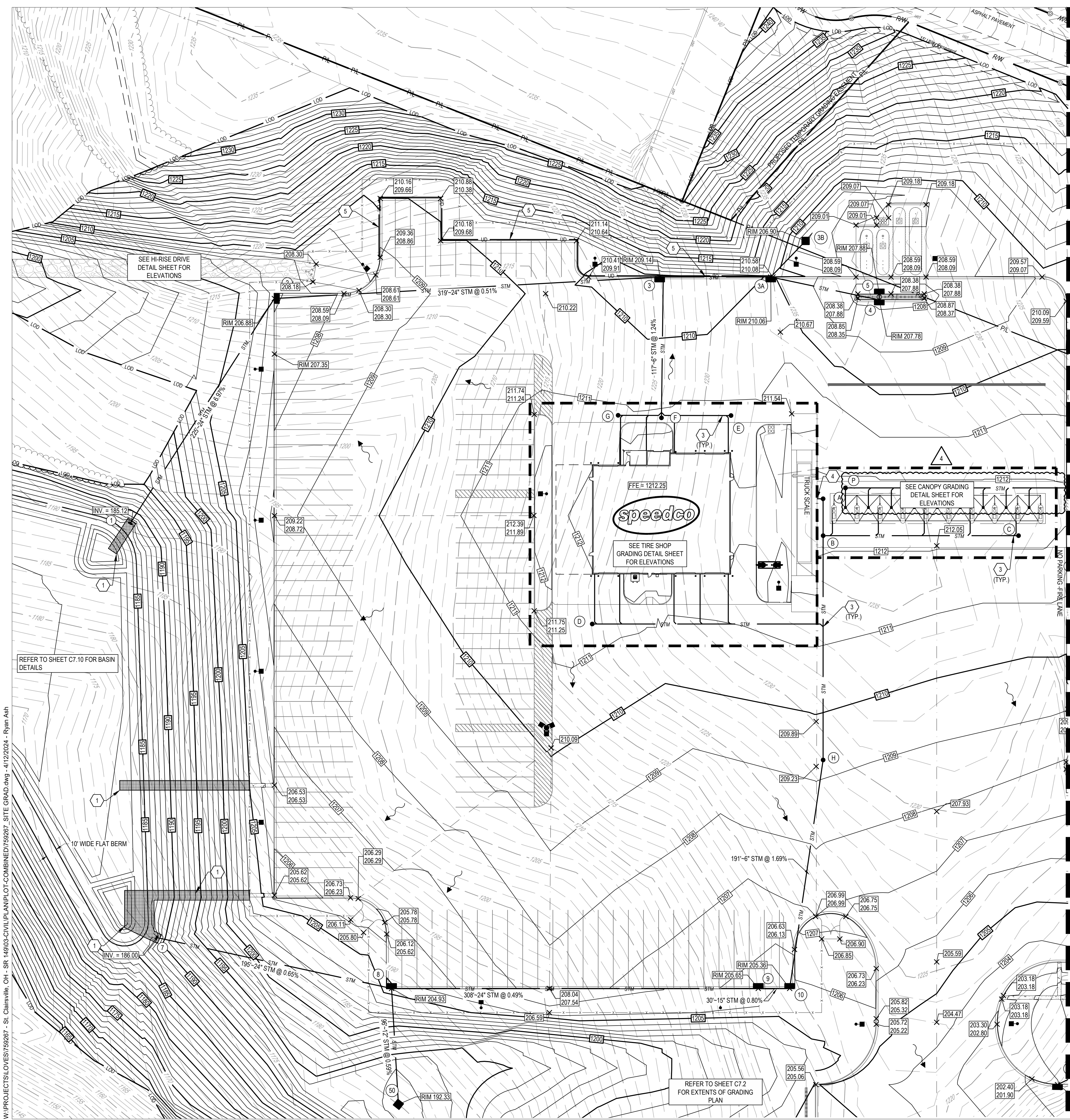
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Issue: OUT TO BID

Drawing Title:
GRADING & DRAINAGE PLAN - EAST

C7.0

V:\PROJECTS\LOVES759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\DOT-COMBINED\759267 SITE GRAD.dwg - 4/12/2024 - Ryan Ash



LEGEND

EXISTING
REFER TO ALTA/SPS TOPOGRAPHIC SURVEY

PROPOSED

- MAJOR CONTOUR
- MINOR CONTOUR
- - - RIDGE LINE
- STM — STORM SEWER
- UD — STORM SEWER UNDERDRAIN
- STORM CURB INLET PER CONSTRUCTION DETAILS
- STORM HALF HEIGHT HEADWALL PER ODOT HW-2.1 DETAIL
- STORM CLEANOUT
- YARD DRAIN
- SANITARY STANDARD MANHOLE PER CONSTRUCTION DETAILS
- SANITARY CLEANOUT
- XXX.XX X SPOT ELEVATION
- XXX.XX X TOP OF CURB
- XXX.XX X BOTTOM OF CURB
- RIM TOP OF CASTING ELEVATION
- RIP RAP
- FLOW ARROW

- GENERAL NOTES**
- ALL WORK AND MATERIALS SHALL COMPLY WITH ALL ODOT REGULATIONS AND CODES, AND O.S.H.A. STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL PERMITTING AND APPROVAL/INSPECTIONS AS REQUIRED FROM THE LOCALITY.
 - THE CONTRACTOR SHALL VERIFY ELEVATIONS AT CONNECTION POINTS OR ANY EXISTING UTILITY CROSSING PRIOR TO ORDERING STRUCTURES. REPORT ANY CONFLICTS TO THE ENGINEER.
 - ALL SPOT ELEVATIONS REFER TO FINISHED PAVEMENT ELEVATION UNLESS OTHERWISE NOTED.
 - PROVIDE 2% MAXIMUM CROSS SLOPE ON SIDEWALKS AND ADA PARKING AREA.
 - ALL CURB AND WALL UNDERDRAINS SHALL OUTLET TO STORM SEWER SYSTEM. PROVIDE POSITIVE DRAINAGE TO CONNECTION POINT.
 - CONTRACTOR SHALL APPLY EROSION CONTROL BLANKET AND LOW MAINTENANCE GRASS SEED MIX TO ALL SLOPES 3H:1V OR STEEPER. REFER TO SWPPP DETAILS FOR LOW MAINTENANCE GRASS SEED MIXTURE SPECIFICATIONS.
 - ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.
 - REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION.
 - PROVIDE STORM SEWER MINIMUM 2'-0" COVER UNLESS OTHERWISE NOTED.
 - PROVIDE FINGER DRAINS AND CONCRETE COLLARS ON ALL PROPOSED STORM SEWER STRUCTURES IN PAVEMENT PER CONSTRUCTION DETAILS SHEET.
 - CONTRACTOR SHALL NOT ALLOW WATER TO POND AT SUBGRADE OR BASE MATERIAL ADJACENT TO CURB INLETS AND CATCH BASINS PRIOR TO PAVEMENT PLACEMENT. TEMPORARY PROVISIONS SUCH AS DEWATERING AND INSTALLATION OF SUBDRAINS SHALL BE TAKEN TO KEEP THE SUBGRADE DRY DURING CONSTRUCTION.
 - ENSURE POSITIVE DRAINAGE AWAY FROM BUILDING(S) AT ALL TIMES. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL DETAILS.

STORM SEWER CLEANOUT SCHEDULE

NO.	STRUCTURE	RIM	INVERT
A	6" CLEANOUT	1212.24	1207.78 (6") S
B	6" CLEANOUT	1212.16	1207.46 (6") N 1207.46 (6") E 1207.46 (6") S
C	6" CLEANOUT	1212.15	1209.15 (6") W
D	6" CLEANOUT	1211.43	1208.64 (6") E
E	6" CLEANOUT	1211.27	1208.19 (6") W
F	6" CLEANOUT	1211.15	1207.59 (6") W 1207.59 (6") E 1207.59 (6") N
G	6" CLEANOUT	1211.11	1207.96 (6") E
H	6" CLEANOUT	1209.29	1205.53 (6") N 1205.53 (6") S
P	6" CLEANOUT	1212.16	1208.69 (6") E

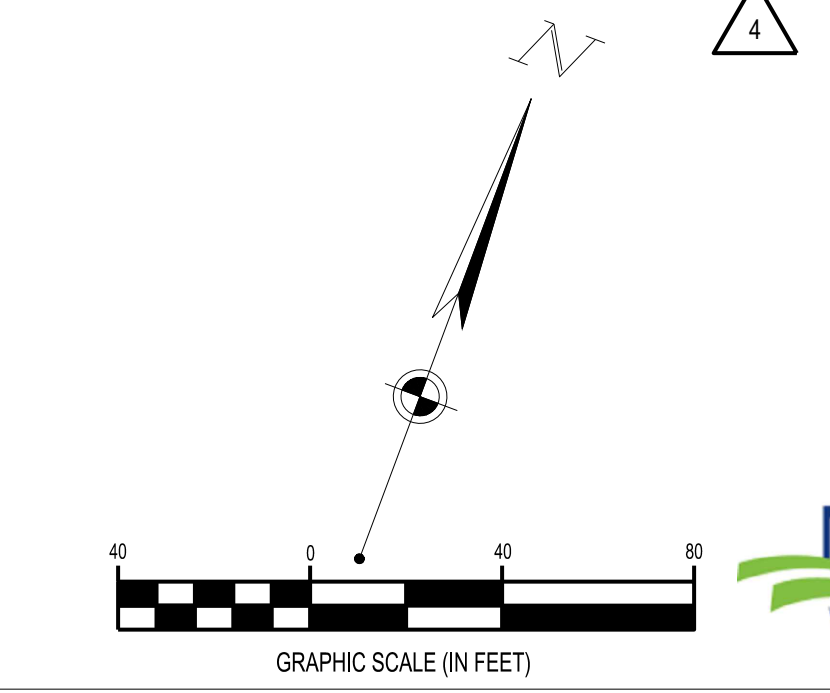
STORM SEWER STRUCTURE SCHEDULE

NO.	STRUCTURE	RIM	INVERT
1	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1		1185.12 (24") N
2	48" INLET MANHOLE	1206.84	1200.79 (24") NE 1200.79 (24") S
3	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1209.14	1202.41 (24") E 1206.14 (6") S 1202.41 (24") SW
3A	48" INLET MANHOLE	1210.06	1202.87 (24") E 1204.35 (12") NE 1202.87 (24") W
3B	2X2 CATCH BASIN	1206.90	1204.56 (12") SW
4	48" INLET MANHOLE	1207.78	1203.83 (18") N 1203.33 (24") W
5	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1207.88	1203.85 (18") S
7	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1		1186.00 (24") E
8	48" INLET MANHOLE	1204.85	1199.06 (24") E 1188.27 (12") SE 1187.27 (24") W
9	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1205.65	1201.32 (15") E 1200.57 (24") W
10	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1205.31	1202.31 (6") N 1201.56 (15") W
48	OUTLET CONTROL STRUCTURE		1180.00 (12") SW
49	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1		1179.50 (12") NE
50	2X2 CATCH BASIN	1192.33	1188.83 (12") NW

- CODED NOTES**
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 - WYE CONNECTION
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 - PROPOSED UNDERDRAIN.
- REFER TO CONSTRUCTION DETAILS SHEET
- REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
- REFER TO SHEETS C7.13 TO C7.15 FOR STORM SEWER PROFILES

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Before You Dig

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ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions

ID	Description	Date
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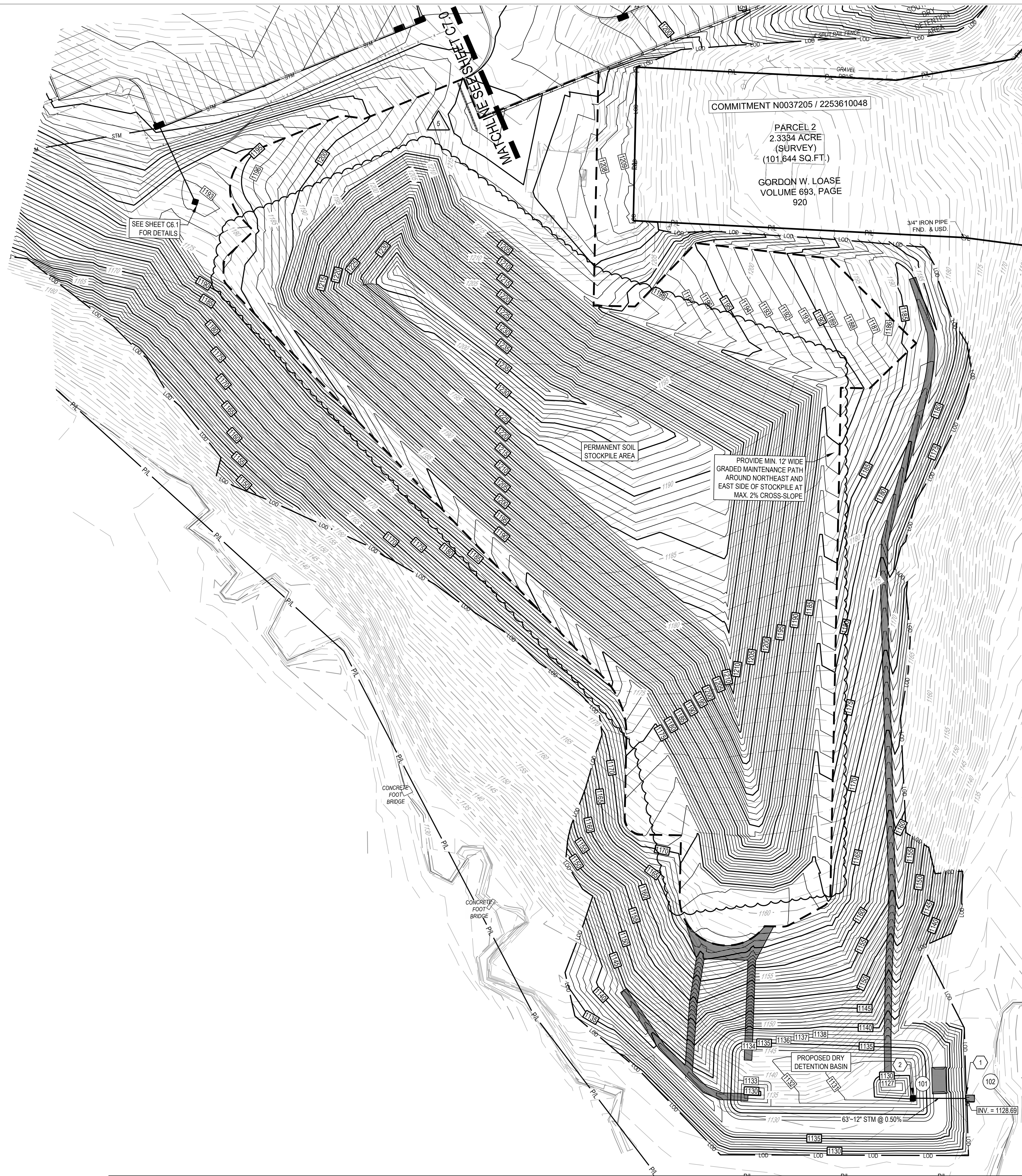
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C7.1

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\DOT-COMBINED\759267 SITE GRAD.dwg - 4/12/2024 - Ryan Ash

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267_RV GRAD OVR.dwg - 5/10/2024 - Robert Hermann



PERMANENT STOCKPILE NOTES:
 CONTRACTOR SHALL PLACE FILL IN SHOWN STOCKPILE AREA AS NEEDED FOR EXCAVATION IN TRUCK STOP AND TRAVEL CENTER AREA. FILL SHALL BE PLACED IN LIFTS AND COMPACTED PER GUIDANCE FROM GEOTECHNICAL ENGINEER. SLOPE OF STOCKPILE SHALL NOT EXCEED 3:1. CONTRACTOR SHALL PLACE A DOUBLE ROW OF SILT FENCE AROUND ENTIRE STOCKPILE AND FULLY STABILIZE STOCKPILE WITH PERMANENT SEEDING UPON FINAL GRADING. CONTRACTOR SHALL GRADE PERIMETER SWALES TO PROPOSED DETENTION FACILITY, AS SHOWN.

- CODED NOTES**
1. RIP RAP. SEE SWPPP FOR DIMENSIONS
 2. OUTLET STRUCTURE PER DETAIL ON SHEET C7.12

LEGEND

EXISTING

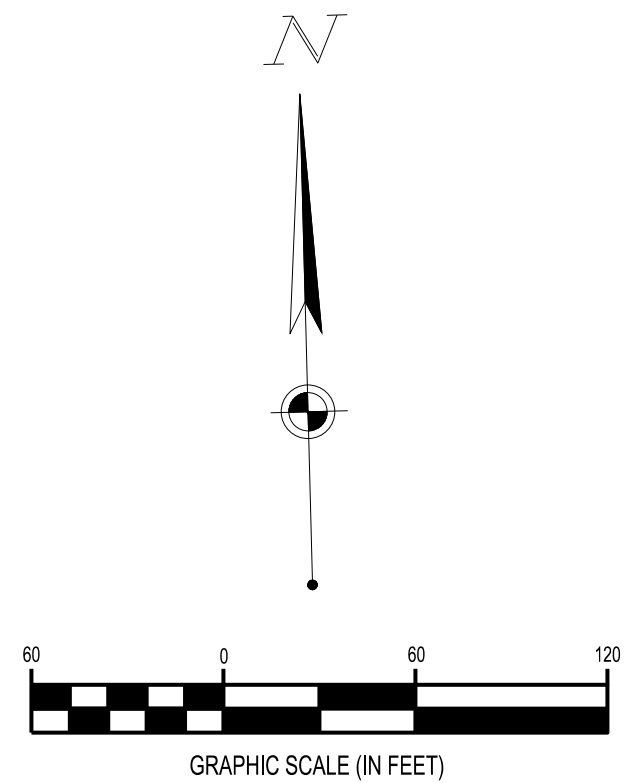
- REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- MAJOR CONTOUR
- MINOR CONTOUR
- RIDGE LINE
- STM
- STORM SEWER
- STORM CATCH BASIN [TYPE]
- STORM ENDWALL
- STORM CLEANOUT
- SPOT ELEVATION
- TRUNCATED SPOT ELEVATION
- FLEXMAT OR APPROVED EQUAL

PROPOSED

- MAJOR CONTOUR
- MINOR CONTOUR
- RIDGE LINE
- STM
- STORM SEWER
- STORM CATCH BASIN [TYPE]
- STORM ENDWALL
- STORM CLEANOUT
- SPOT ELEVATION
- TRUNCATED SPOT ELEVATION
- FLEXMAT OR APPROVED EQUAL

REFER TO CONSTRUCTION DETAILS SHEET
 REFER TO DETAIL GRADING SHEETS C7.0 TO C7.2
 REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
 REFER TO SHEETS C7.13 TO C7.15 FOR STORM SEWER PROFILE

- GENERAL NOTES**
1. ALL WORK AND MATERIALS SHALL COMPLY WITH ALL ODOT REGULATIONS AND CODES, AND O.S.H.A. STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL PERMITTING AND APPROVAL INSPECTIONS AS REQUIRED FROM THE LOCALITY.
 2. THE CONTRACTOR SHALL VERIFY ELEVATIONS AT CONNECTION POINTS OR ANY EXISTING UTILITY CROSSING PRIOR TO ORDERING STRUCTURES. REPORT ANY CONFLICTS TO THE ENGINEER.
 3. LINES UNDERGROUND SHALL BE INSTALLED, INSPECTED, AND APPROVED PRIOR TO BACKFILLING.
 4. THE CONTRACTOR IS RESPONSIBLE FOR THE REPAIRS TO UTILITIES DAMAGED DURING CONSTRUCTION AT NO COST TO THE OWNER.
 5. STORM SEWER CATCH BASINS, CURB INLETS, MANHOLES, AND ENDWALLS SHALL CONFORM TO ODOT STANDARD CONSTRUCTION DRAWINGS.
 6. ALL SPOT ELEVATIONS REFER TO FINISHED PAVEMENT ELEVATION UNLESS OTHERWISE NOTED.
 7. PROVIDE 2% MAXIMUM CROSS SLOPE ON SIDEWALKS AND ADA PARKING AREA.
 8. ALL CURB AND WALL UNDERDRAINS SHALL OUTLET TO STORM SEWER SYSTEM. PROVIDE POSITIVE DRAINAGE TO CONNECTION POINT.
 9. CONTRACTOR SHALL APPLY EROSION CONTROL BLANKET AND LOW MAINTENANCE GRASS SEED MIX TO ALL SLOPES 3H:1V OR STEEPER. REFER TO SWPPP DETAILS FOR LOW MAINTENANCE GRASS SEED MIXTURE SPECIFICATIONS.
 10. ADD 1100.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.
 11. REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION.
 12. PROVIDE STORM SEWER MINIMUM 1'-6" COVER UNLESS OTHERWISE NOTED.
 13. PROVIDE FINGER DRAINS AND CONCRETE COLLARS ON ALL PROPOSED STORM SEWER STRUCTURES IN PAVEMENT PER CONSTRUCTION DETAILS SHEET.
 14. CONTRACTOR SHALL NOT ALLOW WATER TO POND AT SUBGRADE OR BASE MATERIAL ADJACENT TO CURB INLETS AND CATCH BASINS PRIOR TO PAVEMENT PLACEMENT. TEMPORARY PROVISIONS SUCH AS DEWATERING AND INSTALLATION OF SUBDRAINS SHALL BE TAKEN TO KEEP THE SUBGRADE DRY DURING CONSTRUCTION.
 15. ENSURE POSITIVE DRAINAGE AWAY FROM BUILDING(S) AT ALL TIMES. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL DETAILS.



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

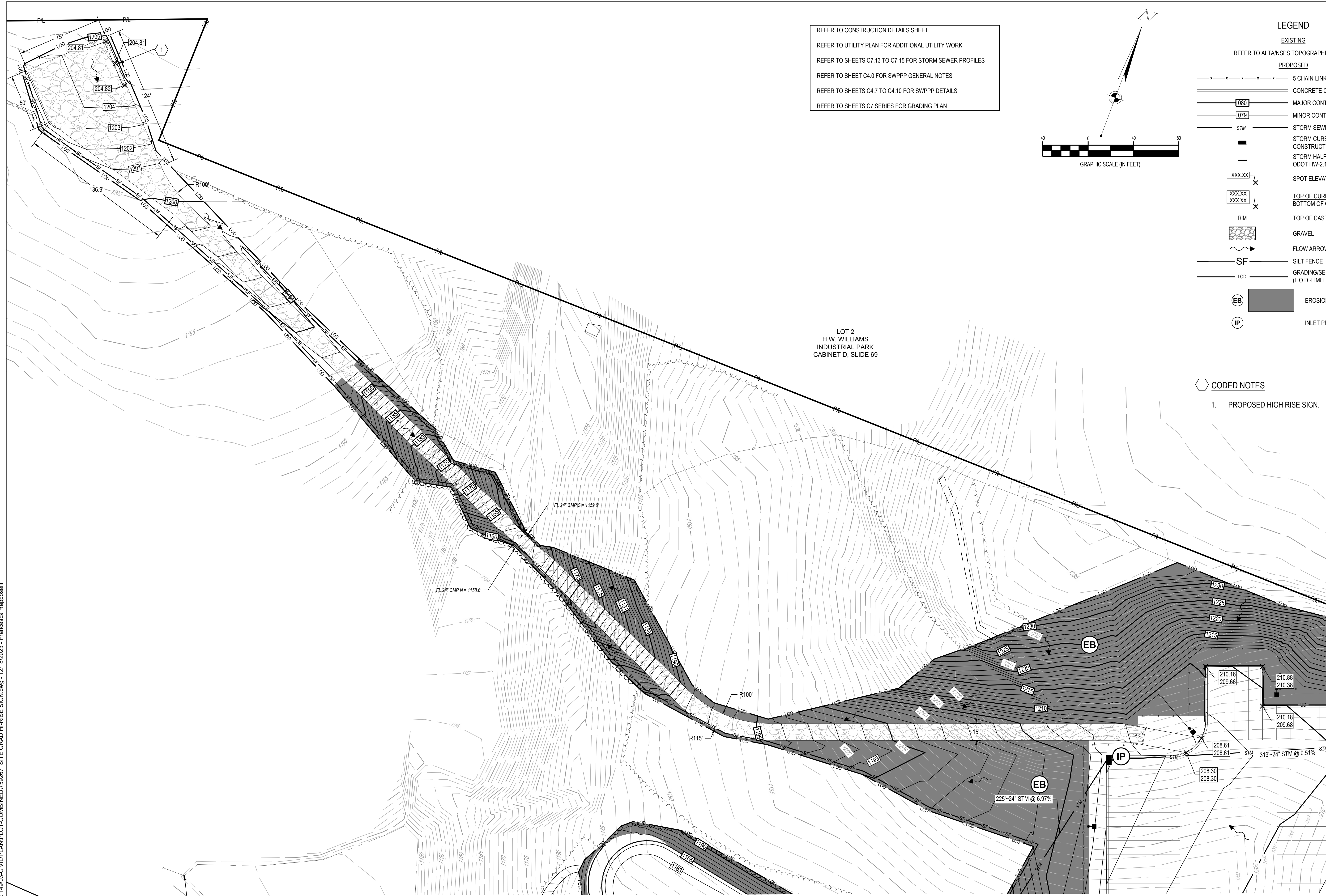
Revisions / Submissions		
ID	Description	Date
5	ADDENDUM 5	04/18/2024

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 Project Number: 759267
 Scale: 1"=60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

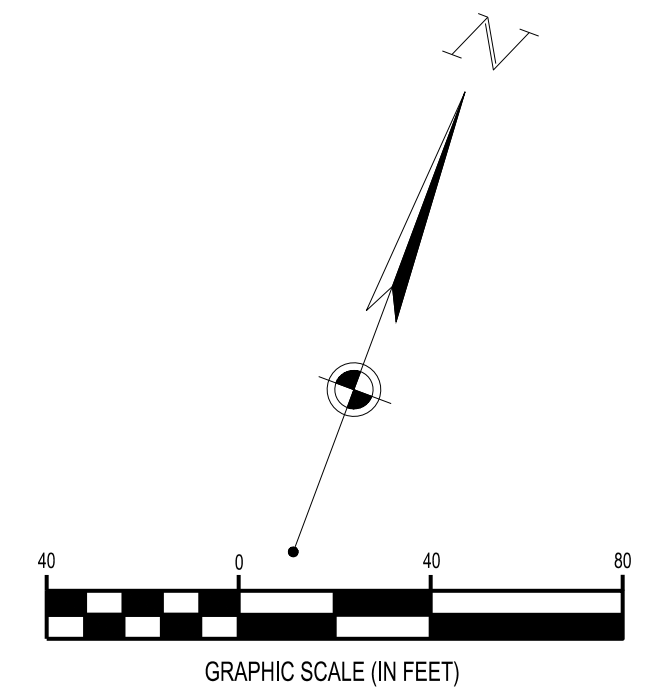
Drawing Title:
**GRADING & DRAINAGE
 PLAN - PREPARED
 PAD**

C7.2

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE GRAD HI-RISE SIGN.dwg - 12/18/2023 - Francesca Rapposelli



REFER TO CONSTRUCTION DETAILS SHEET
 REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
 REFER TO SHEETS C7.13 TO C7.15 FOR STORM SEWER PROFILES
 REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
 REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN



LEGEND	
EXISTING	
REFER TO ALTA/SPS TOPOGRAPHIC SURVEY	
PROPOSED	
	5 CHAIN-LINK FENCE
	CONCRETE CURB AND GUTTER
	MAJOR CONTOUR
	MINOR CONTOUR
	STORM SEWER
	STORM CURB INLET PER CONSTRUCTION DETAILS
	STORM HALF HEIGHT HEADWALL PER ODOT HW-2.1 DETAIL
	SPOT ELEVATION
	TOP OF CURB
	BOTTOM OF CURB
	TOP OF CASTING ELEVATION
	GRAVEL
	FLOW ARROW
	SILT FENCE
	GRADING/SEEDING LIMITS (L.O.D.-LIMIT OF DISTURBANCE)
	EROSION CONTROL BLANKET
	INLET PROTECTION

CODED NOTES
 1. PROPOSED HIGH RISE SIGN.

LOT 2
 H.W. WILLIAMS
 INDUSTRIAL PARK
 CABINET D, SLIDE 69



175 Monroe West Ave., Suite 400
 Akron, OH 44321
 Phone: 330.665.0960 Fax: 888.208.4826



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
ID	Description	Date

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 Project Number: 759267
 Scale: 1" = 40'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
HIGH RISE DRIVE SITE & SWPPP PLAN

C7.3



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

GENERAL NOTES

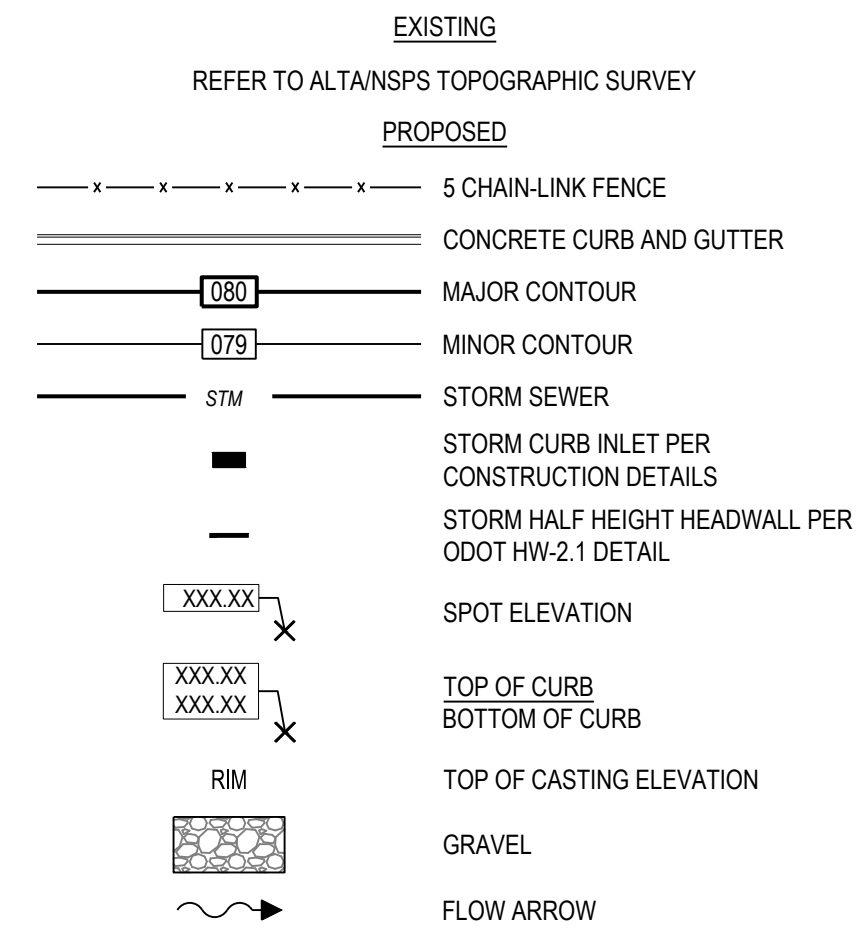
- ALL WORK AND MATERIALS SHALL COMPLY WITH ALL ODOT REGULATIONS AND CODES, AND O.S.H.A. STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL PERMITTING AND APPROVAL/INSPECTIONS AS REQUIRED FROM THE LOCALITY.
- THE CONTRACTOR SHALL VERIFY ELEVATIONS AT CONNECTION POINTS OR ANY EXISTING UTILITY CROSSING PRIOR TO ORDERING STRUCTURES. REPORT ANY CONFLICTS TO THE ENGINEER.
- LINES UNDERGROUND SHALL BE INSTALLED, INSPECTED, AND APPROVED PRIOR TO BACKFILLING.
- THE CONTRACTOR IS RESPONSIBLE FOR THE REPAIRS TO UTILITIES DAMAGED DURING CONSTRUCTION AT NO COST TO THE OWNER.
- STORM SEWER CATCH BASINS, CURB INLETS, MANHOLES, AND ENDWALLS SHALL CONFORM TO ODOT STANDARD CONSTRUCTION DRAWINGS.
- ALL SPOT ELEVATIONS REFER TO FINISHED PAVEMENT ELEVATION UNLESS OTHERWISE NOTED.
- PROVIDE 2% MAXIMUM CROSS SLOPE ON SIDEWALKS AND ADA PARKING AREA.
- ALL CURB AND WALL UNDERDRAINS SHALL OUTLET TO STORM SEWER SYSTEM. PROVIDE POSITIVE DRAINAGE TO CONNECTION POINT.
- CONTRACTOR SHALL APPLY EROSION CONTROL BLANKET AND LOW MAINTENANCE GRASS SEED MIX TO ALL SLOPES 3H:1V OR STEEPER. REFER TO SWPPP DETAILS FOR LOW MAINTENANCE GRASS SEED MIXTURE SPECIFICATIONS.
- ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.
- REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION.
- PROVIDE STORM SEWER MINIMUM 2'-0" COVER UNLESS OTHERWISE NOTED.
- PROVIDE FINGER DRAINS AND CONCRETE COLLARS ON ALL PROPOSED STORM SEWER STRUCTURES IN PAVEMENT PER CONSTRUCTION DETAILS SHEET.
- CONTRACTOR SHALL NOT ALLOW WATER TO POND AT SUBGRADE OR BASE MATERIAL ADJACENT TO CURB INLETS AND CATCH BASINS PRIOR TO PAVEMENT PLACEMENT. TEMPORARY PROVISIONS SUCH AS DEWATERING AND INSTALLATION OF SUBDRAINS SHALL BE TAKEN TO KEEP THE SUBGRADE DRY DURING CONSTRUCTION.
- ENSURE POSITIVE DRAINAGE AWAY FROM BUILDING(S) AT ALL TIMES. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL DETAILS.

REFER TO CONSTRUCTION DETAILS SHEET
 REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
 REFER TO SHEETS C7.13 TO C7.12 FOR STORM SEWER PROFILES
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN

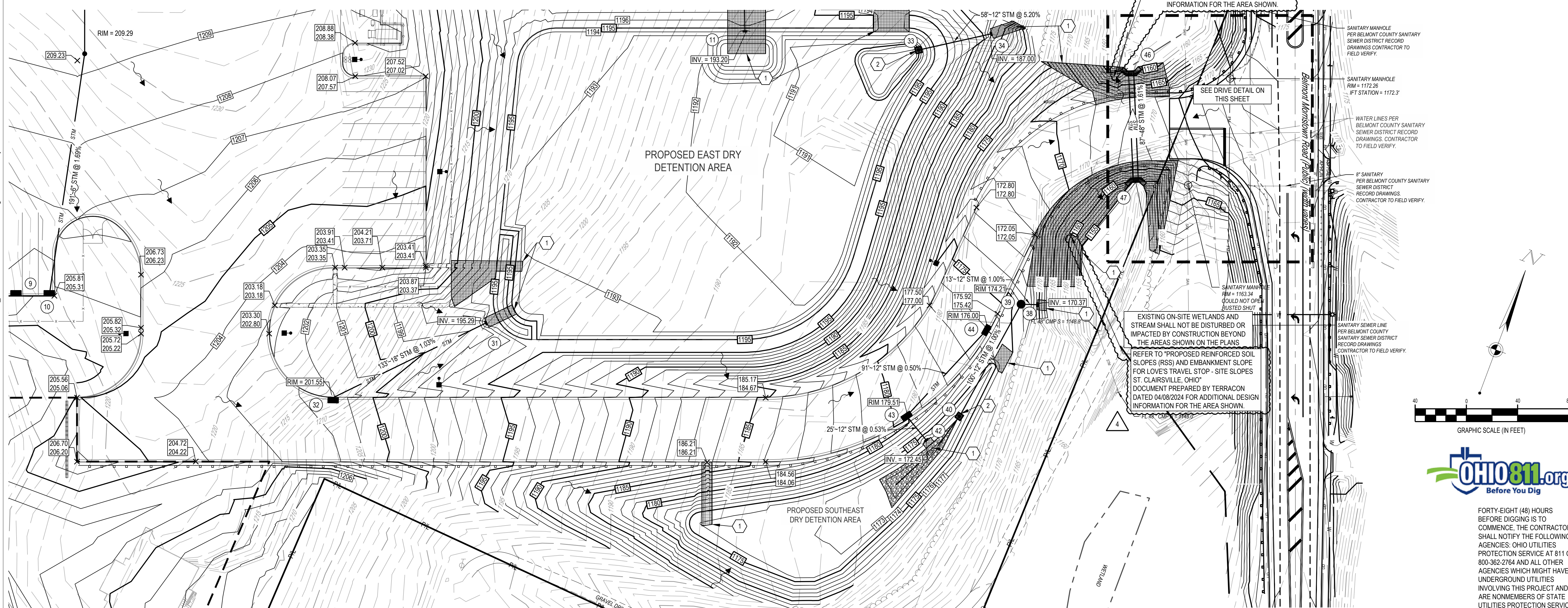
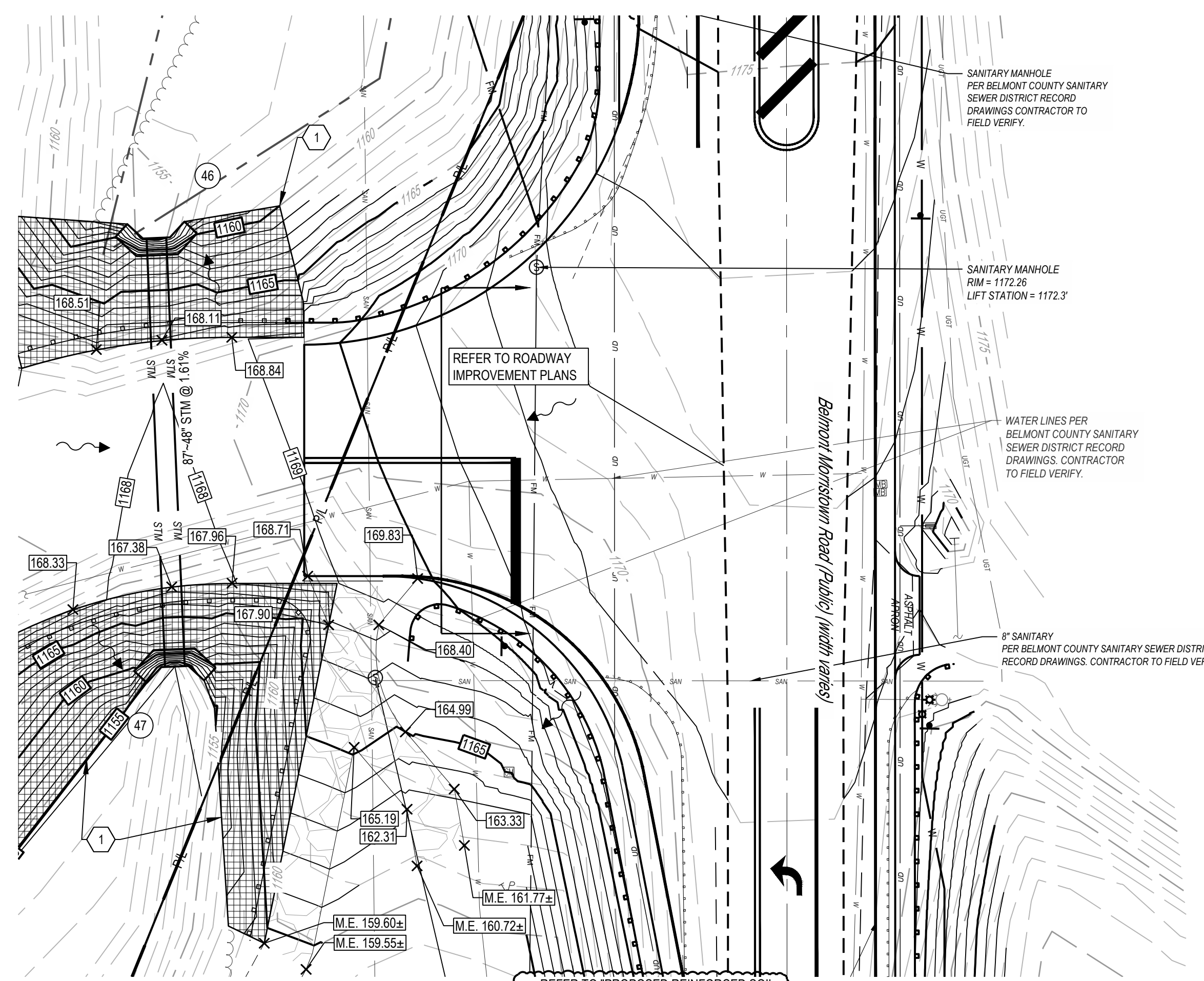
CODED NOTES

- CONTRACTOR TO INSTALL EROSION CONTROL FLEXAMAT STANDARD OR APPROVED EQUAL
- OUTLET STRUCTURE. REFER TO SHEETS C7.7 TO C7.9.

LEGEND



STORM SEWER STRUCTURE SCHEDULE			
NO.	STRUCTURE	RIM	INVERT
11	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1193.20 (18") N	
31	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1195.29 (18") SW	
32	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1201.55	1196.66 (18") NE
33	OUTLET CONTROL STRUCTURE	1190.00 (12") NE	
34	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1187.00 (12") SW	
38	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1170.37 (12") W	
39	48" STORM MANHOLE	1174.21	1170.50 (12") S 1170.50 (12") E
40	OUTLET CONTROL STRUCTURE	1171.50 (12") N	
42	ODOT HALF-HEIGHT HEADWALL PER STD. DWG. HW-2.1	1172.45 (12") NW	
43	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1179.54	1172.58 (12") N 1172.58 (12") SE
44	3' X 3' PRECAST CURB INLET PER CONSTRUCTION DETAILS	1176.04	1173.04 (12") S
46	ODOT FULL-HEIGHT HEADWALL PER STD. DWG. HW-1.1	1154.00 (48") SE	
47	ODOT FULL-HEIGHT HEADWALL PER STD. DWG. HW-1.1	1152.60 (48") NW	



LOVE'S TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions		
ID	Description	Date
1	REV 1	01/03/2024
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
 Scale: 1" = 40'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID



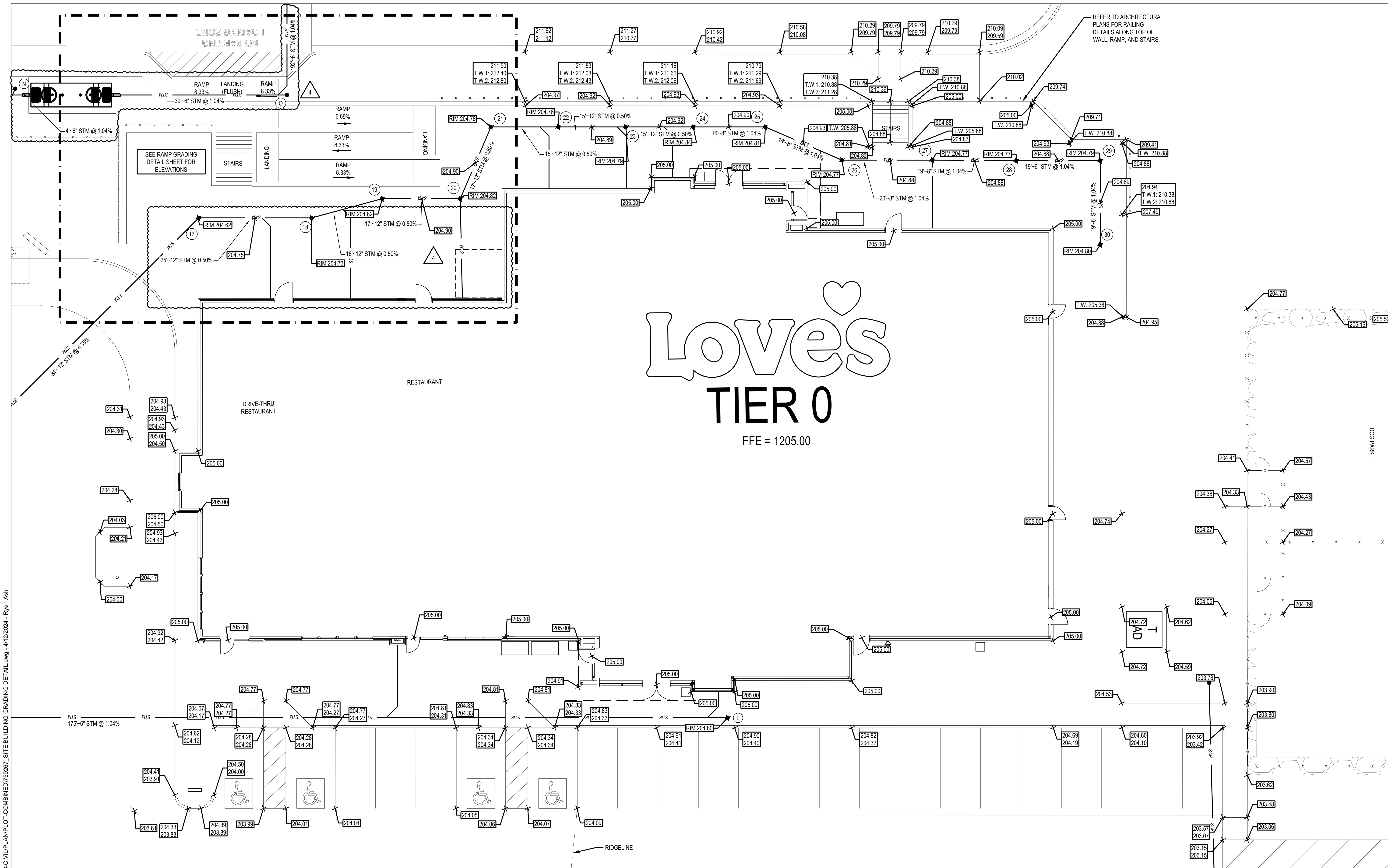
FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

Drawing Title:
SOUTH DRIVE DETAIL

C7.4

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE GRAD TRUCK DRIVE.dwg - 4/12/2024 - Ryan Ash

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE BUILDING GRADING DETAIL.dwg - 4/12/2024 - Ryan Ash

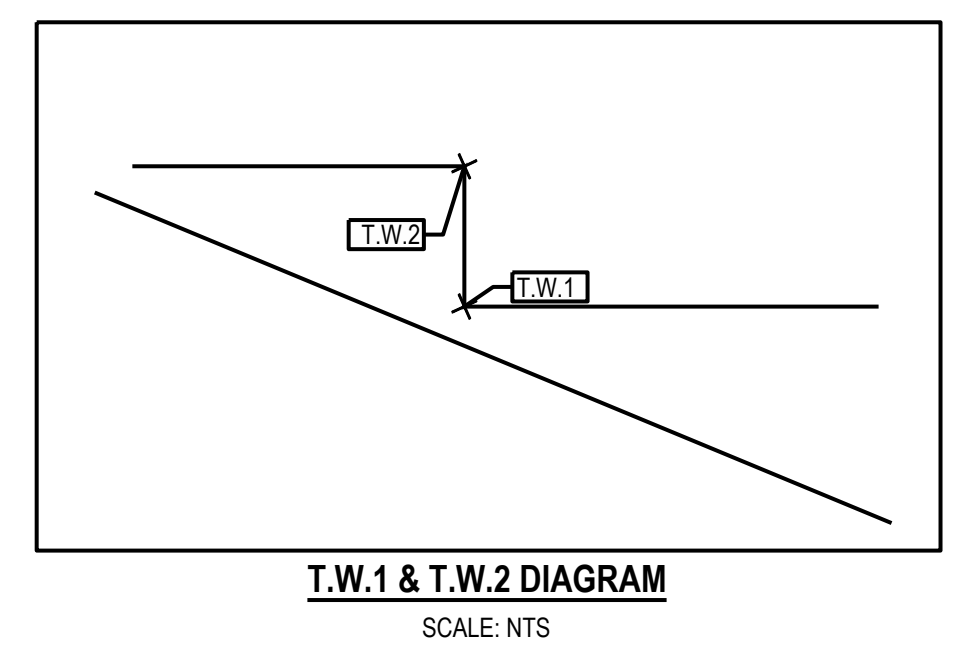


Love's

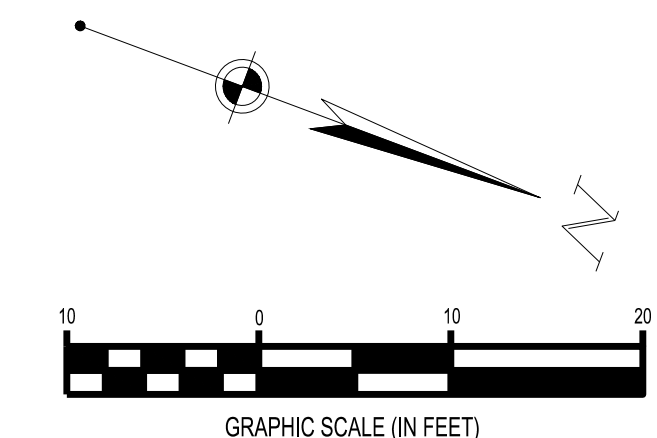
TIER 0

FFE = 1205.00

- NOTES**
1. ALL SPOT GRADES REFER TO TOP OF PAVEMENT UNLESS OTHERWISE NOTED.
 2. SEE GRADING PLAN FOR ADDITIONAL GRADES ON SITE.
 3. ALL DISCREPANCIES BETWEEN SPOT ELEVATIONS ON THIS SHEET AND THE GRADING PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE AND THE DESIGN ENGINEER.
 4. SLOPES IN ADA PARKING AREA AND ACCESSIBLE AISLE WAY SHALL NOT EXCEED 2.0% IN ANY DIRECTION.
 5. ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.



SPOT ELEVATIONS FOR BUILDING
SCALE: 1" = 10'



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

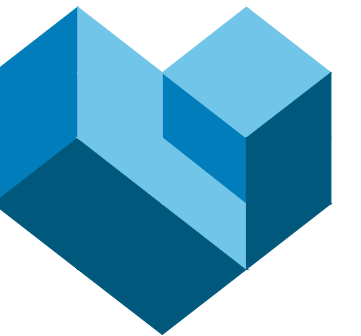
LOVE'S TRAVEL STOP
ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
ID	Description	Date
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
Scale: 1" = 10'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
BUILDING GRADING DETAIL

C7.5



CESO
WWW.CESOINC.COM

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Phone: 330.665.0660 Fax: 888.208.4826



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions

ID	Description	Date
4	ADDENDUM 4	02/23/2024

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Project Number: 759267

Scale: 1"=5'

Drawn By: FAR

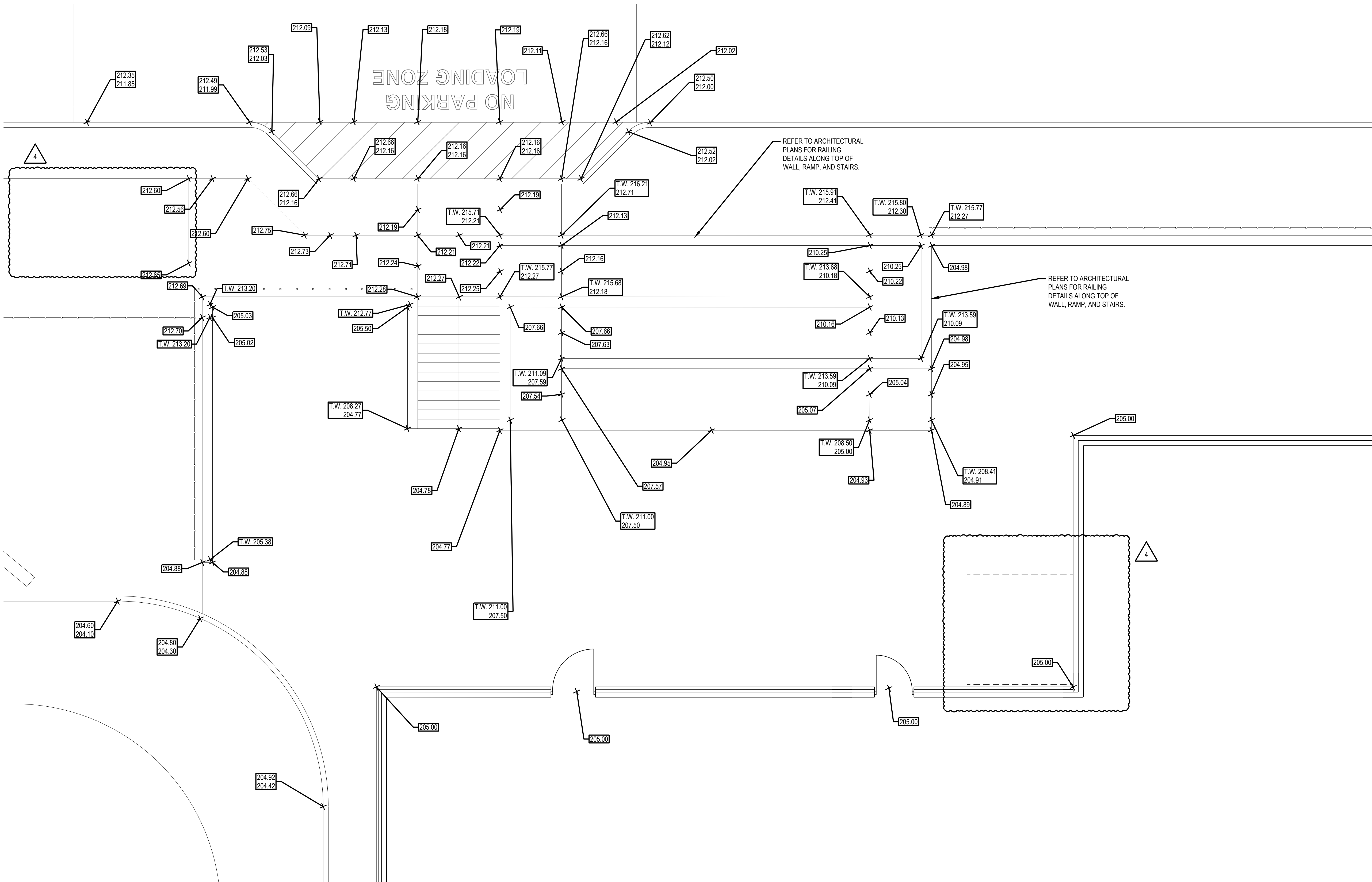
Checked By: JTK

Date: 12/19/2023

Issue: OUT TO BID

Drawing Title:
**RAMP GRADING
DETAIL**

C7.6



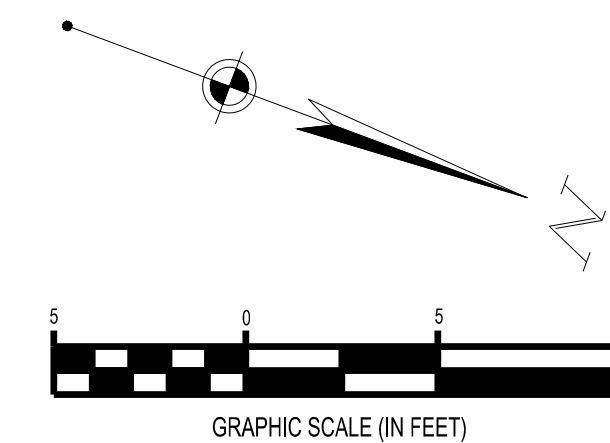
SPOT ELEVATIONS FOR RAMP
SCALE: 1" = 5'

NOTES

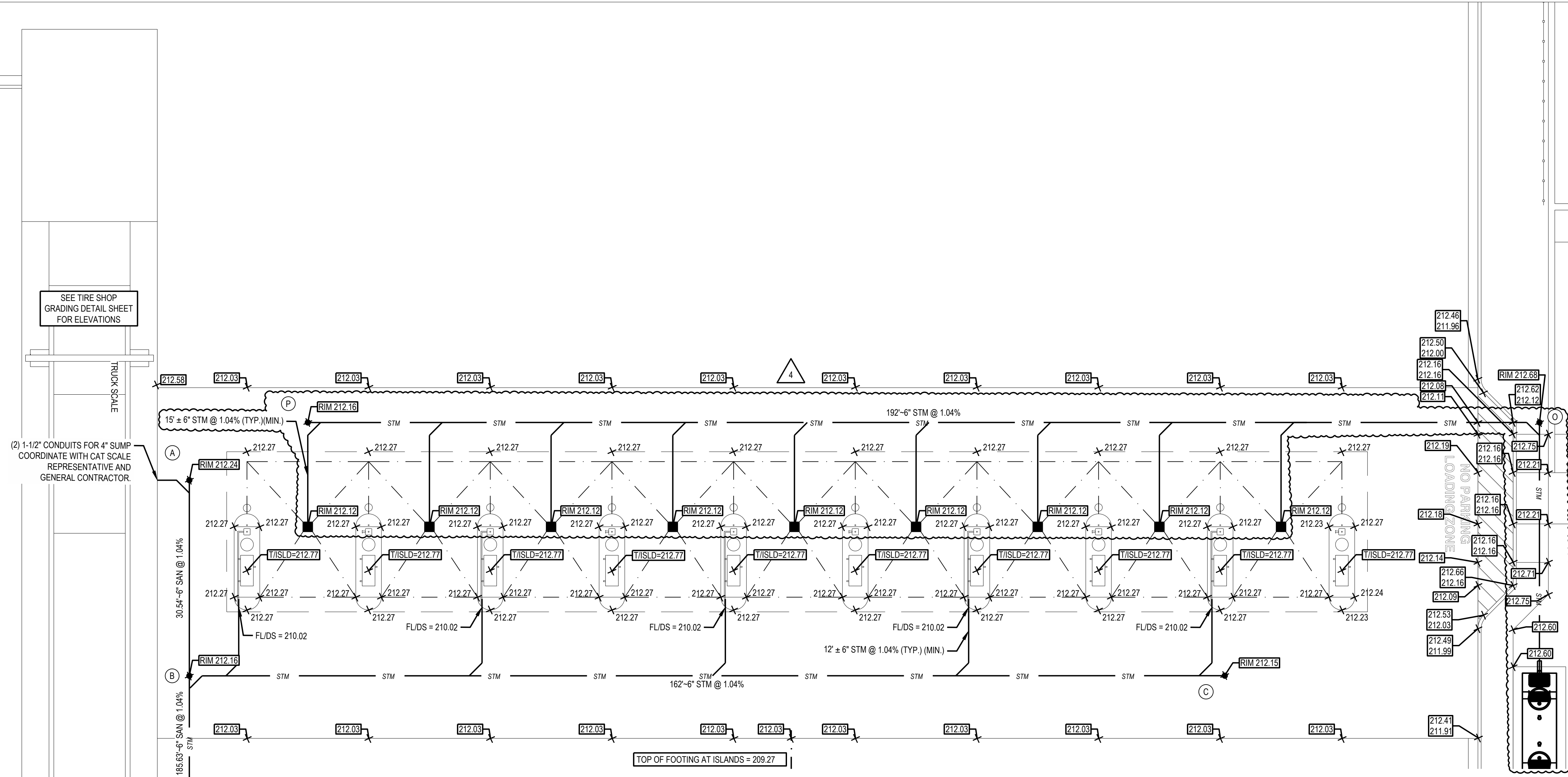
1. ALL SPOT GRADES REFER TO TOP OF PAVEMENT UNLESS OTHERWISE NOTED.
2. SEE GRADING PLAN FOR ADDITIONAL GRADES ON SITE.
3. ALL DISCREPANCIES BETWEEN SPOT ELEVATIONS ON THIS SHEET AND THE GRADING PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE AND THE DESIGN ENGINEER.
4. SLOPES IN ADA PARKING AREA AND ACCESSIBLE AISLE WAY SHALL NOT EXCEED 2.0% IN ANY DIRECTION.
5. ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLANS\DOT-COMBINED\759267 SITE BUILDING GRADING DETAIL.dwg - 4/12/2024 - Francesca Rapposelli

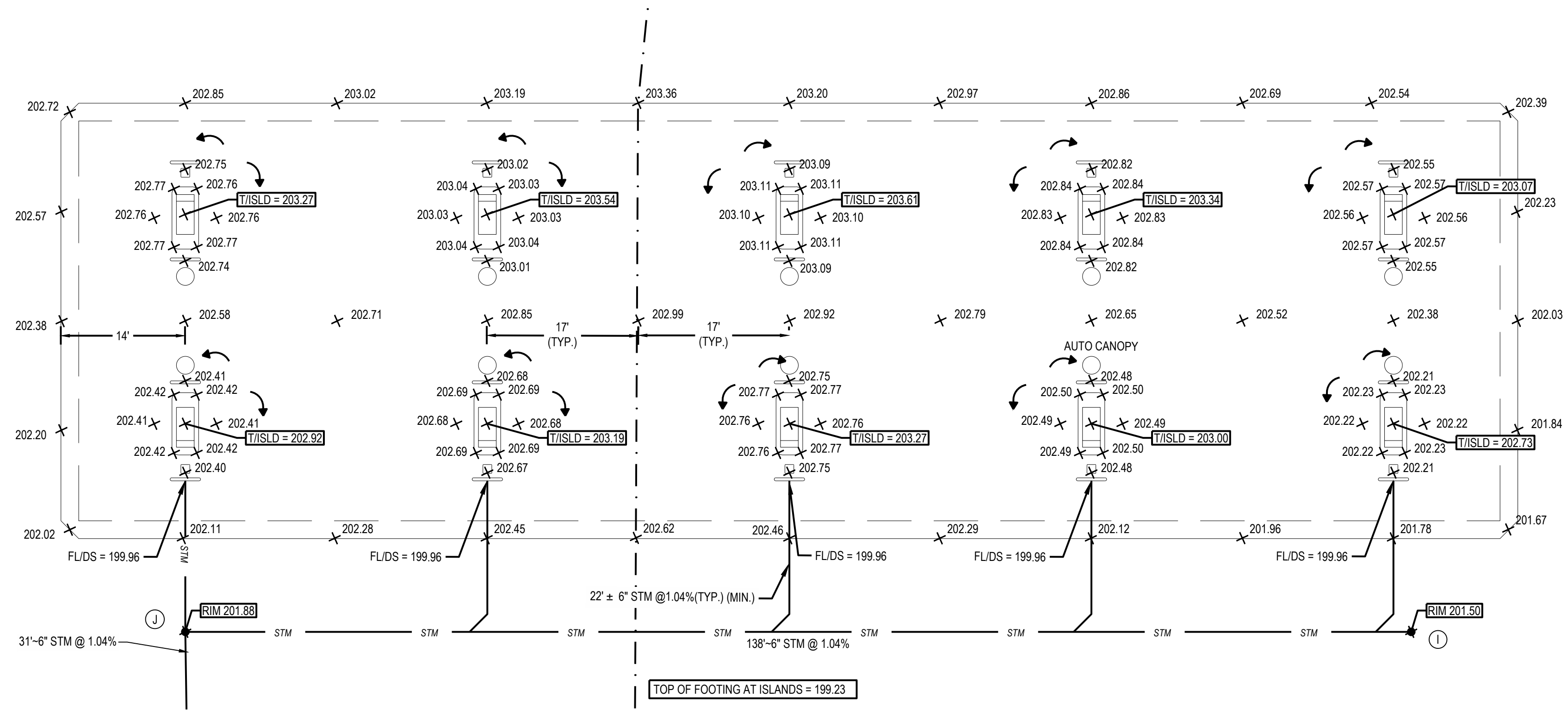
FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\DOT-COMBINED\759267 SITE CANOPY GRADING DETAIL.dwg - 4/12/2024 - Ryan Ash



SPOT ELEVATIONS FOR DIESEL ISLAND
SCALE: 1" = 10'



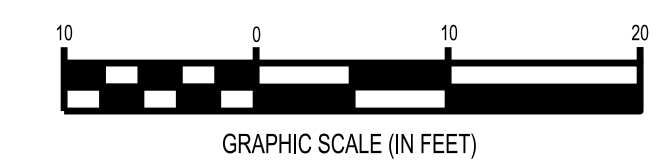
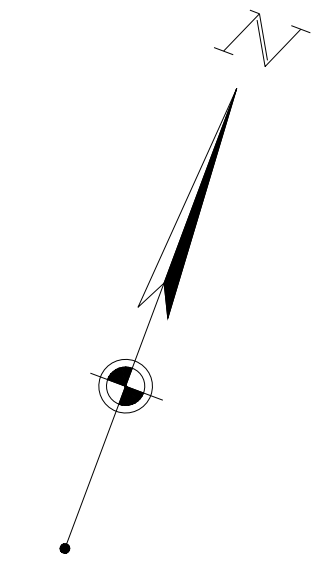
SPOT ELEVATIONS FOR GASOLINE ISLAND
SCALE: 1" = 10'

- NOTES**
- GRADING DESIGN FOR FINISHED PAVEMENT GRADES IS SHOWN HEREIN.
 - STRUCTURAL SUPPORT FOR THE CANOPIES AND DESIGN FOR CANOPY FOOTINGS ARE DESIGNED BY OTHERS AND SHOWN HERE FOR INFORMATIONAL PURPOSES ONLY.
 - FUEL SYSTEM IS DESIGNED BY OTHERS. THE FUEL SYSTEM PLAN LAYOUT IS SHOWN FOR INFORMATIONAL PURPOSES ONLY.
 - CONTRACTOR SHALL ENSURE THAT THE CURB HEIGHT AT THE FUEL DISPENSER ISLAND IS 6 INCHES MINIMUM AND 8 INCHES MAXIMUM.
 - REFERENCE PAVEMENT DESIGNS ARE SHOWN ON CONSTRUCTION DETAILS SHEET.
 - REFER TO UTILITY PLAN FOR SANITARY SEWER STRUCTURE SCHEDULE.
 - ALL SPOT GRADES REFER TO TOP OF PAVEMENT UNLESS OTHERWISE NOTED.
 - SEE GRADING PLAN FOR ADDITIONAL GRADES ON SITE.
 - ALL DISCREPANCIES BETWEEN SPOT ELEVATIONS ON THIS SHEET AND THE GRADING PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE AND THE DESIGN ENGINEER.
 - ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.

LEGEND OF ABBREVIATIONS

T/ISLD - TOP OF PUMP ISLAND
RIM - TOP OF CASTING
FL/DS - FLOW LINE OF PIPE AT COLUMN

REFER TO CONSTRUCTION DETAILS SHEET
REFER TO UTILITY PLAN FOR ADDITIONAL UTILITY WORK
REFER TO SHEETS C7.13 TO C7.15 FOR STORM SEWER PROFILES



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions		
ID	Description	Date
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
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Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
CANOPY GRADING DETAIL

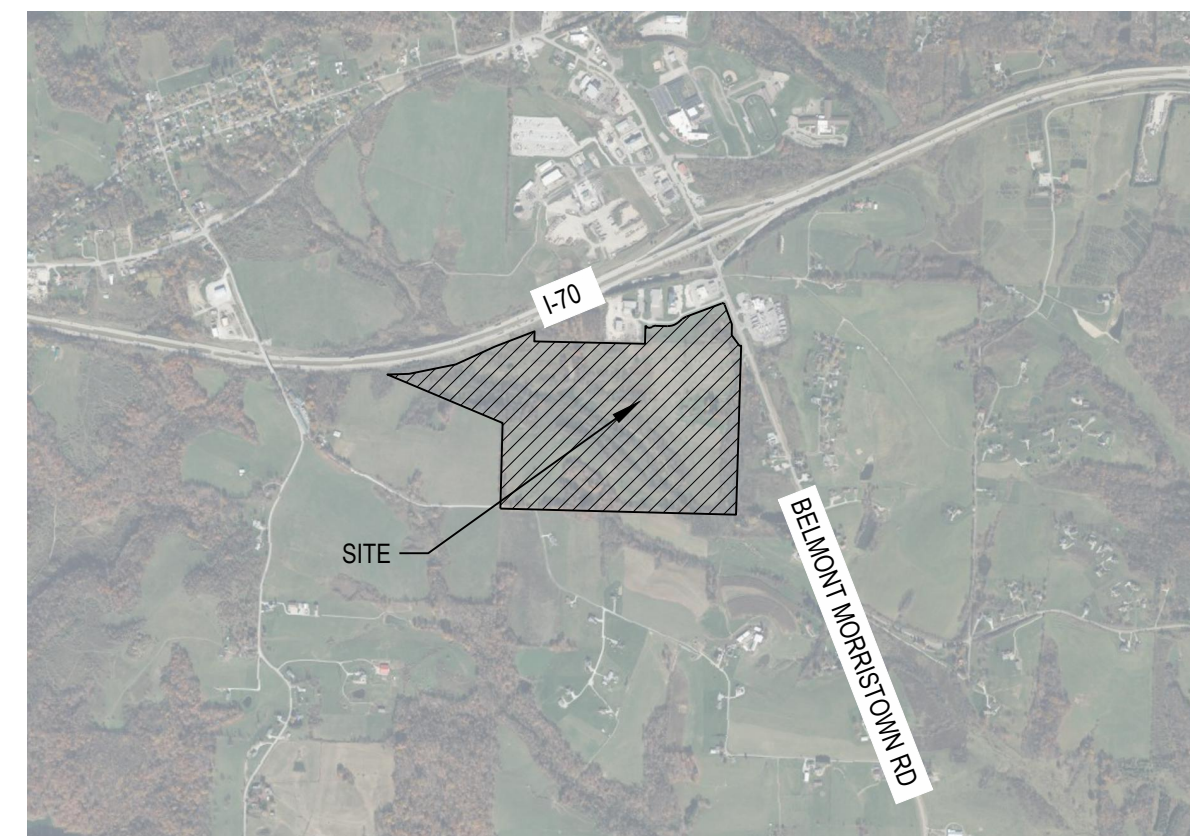
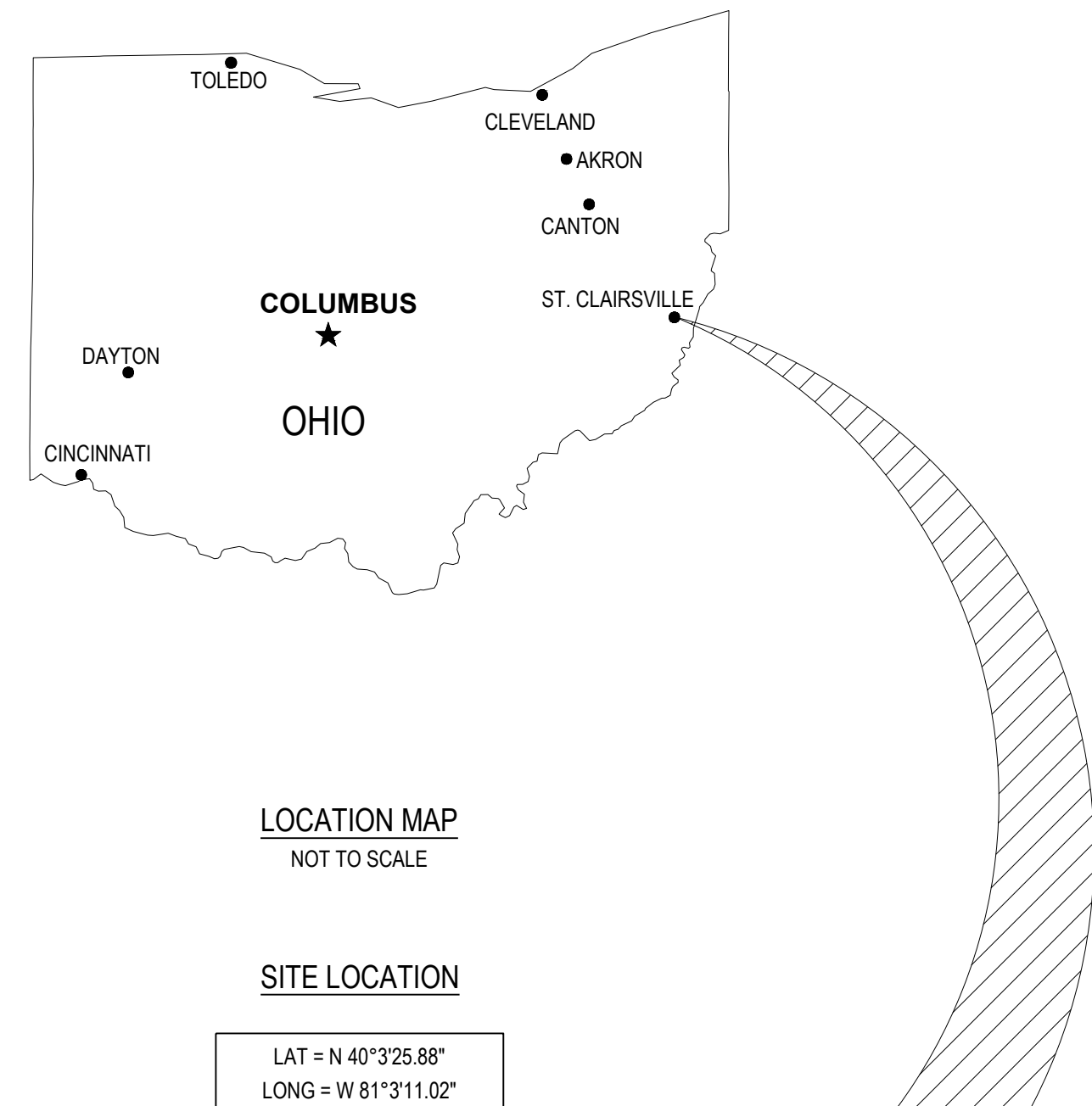
C7.7

PROPOSED LOVE'S TRAVEL STOP

66320 BELMONT-MORRISTOWN ROAD

UNION TOWNSHIP

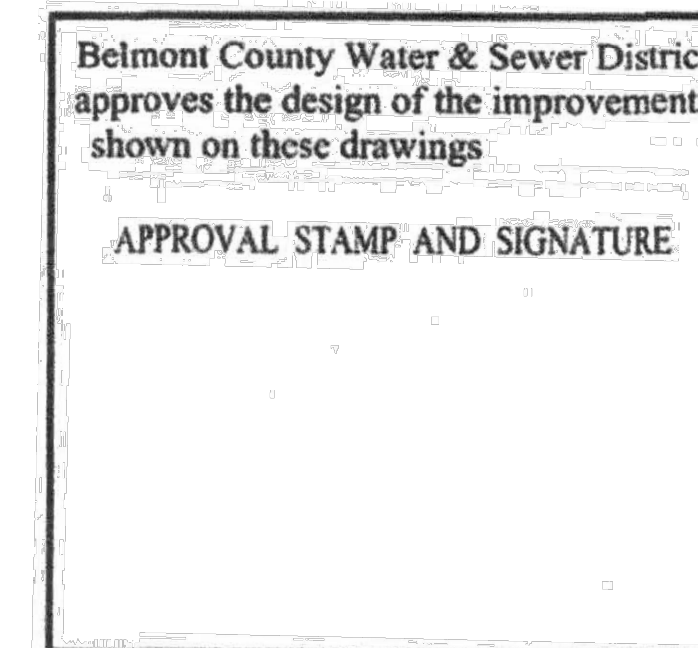
BELMONT COUNTY, OHIO, 43718



Sheet Number	Sheet Title
C1.0	COVER SHEET
C1.1	GENERAL NOTES
C2.0	DEMOLITION PLAN - EAST
C2.1	DEMOLITION PLAN - WEST
C2.2	DEMOLITION PLAN - PREPARED PAD AREA
C3.0	OVERALL SITE PLAN
C3.1	OVERALL SITE PLAN - PREPARED PAD AREA
C4.0	SWPPP NOTES
C4.1	SWPPP PHASE 1
C4.2	SWPPP PHASE 2
C4.3	SWPPP PHASE 3
C4.4	SWPPP PHASE 1 - PREPARED PAD AREA
C4.5	SWPPP PHASE 2 - PREPARED PAD AREA
C4.6	SWPPP PHASE 3 - PREPARED PAD AREA
C4.7	SWPPP DETAILS
C4.8	SWPPP DETAILS
C4.9	SWPPP DETAILS
C4.10	SWPPP DETAILS
C5.0	SITE PLAN - EAST
C5.1	SITE PLAN - WEST
C6.0	PAVEMENT PLAN - EAST
C6.1	PAVEMENT PLAN - WEST
C7.0	GRADING & DRAINAGE PLAN - EAST
C7.1	GRADING & DRAINAGE PLAN - WEST
C7.2	GRADING & DRAINAGE PLAN - PREPARED PAD
C7.3	HIGH RISE DRIVE SITE & SWPPP PLAN
C7.4	SOUTH DRIVE DETAIL
C7.5	BUILDING GRADING DETAIL
C7.6	RAMP GRADING DETAIL
C7.7	CANOPY GRADING DETAIL
C7.8	TIRE SHOP GRADING DETAIL
C7.9	DETENTION DETAILS EAST
C7.10	DETENTION DETAILS WEST
C7.11	DETENTION DETAILS SOUTHEAST
C7.12	DETENTION DETAILS PREPARED PAD
C7.13	STORM PROFILE
C7.14	STORM PROFILE
C7.15	STORM PROFILE
C8.0	UTILITY PLAN - EAST
C8.1	UTILITY PLAN - WEST
C8.2	SANITARY PROFILE
C8.3	SANITARY PROFILE
C9.0	CONSTRUCTION DETAILS
C9.1	CONSTRUCTION DETAILS
C9.2	CONSTRUCTION DETAILS
C9.3	CONSTRUCTION DETAILS
C9.4	CONSTRUCTION DETAILS
C9.5	CONSTRUCTION DETAILS
C9.6	CONSTRUCTION DETAILS
C9.7	CONSTRUCTION DETAILS
L1.0	OVERALL LANDSCAPE PLAN
L1.1	LANDSCAPE PLAN
L1.2	LANDSCAPE PLAN
L1.3	LANDSCAPE PLAN
L1.4	LANDSCAPE PLAN
L2.0	IRRIGATION PLAN
L3.0	PLANTING DETAILS AND NOTES
L3.1	IRRIGATION DETAILS AND NOTES
L3.2	IRRIGATION DETAILS AND NOTES

Sheet Number	Sheet Title
SURVEY-CESO, INC.	
SUR1	ALTA / NSPS LAND TITLE SURVEY
SUR2	ALTA / NSPS LAND TITLE SURVEY
SUR3	ALTA / NSPS LAND TITLE SURVEY
SUR4	ALTA / NSPS LAND TITLE SURVEY

FOR ROADWAY IMPROVEMENTS, REFER TO OH-149 ROADWAY IMPROVEMENT AND SIGNAL PLANS BY CESO, INC.



A DETAILED GEOTECHNICAL REPORT HAS BEEN PREPARED FOR THIS SITE AND SHOULD BE ADHERED TO THE FULL EXTENT OF THOSE RECOMMENDATIONS AND FIELD TESTING. REFER TO GEOTECHNICAL ENGINEERING REPORT PREPARED BY TERRACON PROJECT NO 4225529

OWNER/DEVELOPER
LOVE'S TRAVEL STOPS & COUNTRY STORES, INC.
10601 N. PENNSYLVANIA AVE.
OKLAHOMA CITY, OK 73120

CONTACT: CHAD BRUNER
PH: (405) 463-8801
EMAIL: CHAD.BRUNER@LOVES.COM

ENGINEER
CESO, INC.
175 MONTROSE WEST AVE. SUITE 400
AKRON, OHIO 44321

CONTACT: KIMBERLY COOPER, P.E.
PH: (330) 396-5682
EMAIL: COOPER@CESOINC.COM

STORM SEWER
ODOT
2201 REISER AVENUE SE
NEW PHILADELPHIA, OH 44663
PH: (330) 308-7861
CONTACT: JEREMY CESSNA

EROSION CONTROL
OEPA - SEDO
2195 EAST FRONT STREET
LOGAN, OH 43138
PH: (740) 385-8501

SANITARY SEWER
BELMONT COUNTY WATER & SEWER DISTRICT
67711 OAK VIEW DRIVE
ST. CLAIRSVILLE, OH 43950
PH: (740) 695-3144
CONTACT: JEANNE KINNEY

GAS
COLUMBIA GAS OF OHIO
300 LURAY DRIVE
WINTERSVILLE, OH 43953
PH: (419) 575-2763
CONTACT: ADAM STUPAK

WATER
BELMONT COUNTY WATER & SEWER DISTRICT
67711 OAK VIEW DRIVE
ST. CLAIRSVILLE, OH 43950
PH: (740) 695-3144
CONTACT: JEANNE KINNEY

FIRE
LAFFERTY VOLUNTEER FIRE DEPARTMENT
70191 IRWIN ROAD
LAFFERTY, OH 43951
PH: (740) 310-0901
CONTACT: DUSTIN HUDAK

ROADWAY
STATE DOT:
ODOT - DISTRICT 11
2201 REISER AVE.
NEW PHILADELPHIA, OH 44663
PH: (330) 308-7861
CONTACT: JEREMY CESSNA

ELECTRIC
AMERICAN ELECTRIC POWER (AEP)
47687 NATIONAL RD. W
ST. CLAIRSVILLE, OH 43950
PH: (740) 238-3630
CONTACT: BRENT PANEPLUCCI

LOCAL DOT:
UNION TWP.
PH: (740) 782-1337
CONTACT: JUSTIN DEMARCHI

HEALTH DEPARTMENT
BELMONT COUNTY HEALTH DEPARTMENT
68501 BANNOCK ROAD
ST. CLAIRSVILLE, OH 43950
PH: (740) 695-1202

COMMUNICATIONS
COMCAST
PH: (412) 337-1049
CONTACT: ANGELA MEYERS

NOTE: TELEPHONE AND COMMUNICATIONS CONTACT INFORMATION IS PROVIDED FOR UTILITY CONFLICT COORDINATION PURPOSES ONLY. FOR ANY NECESSARY SERVICE OR ACCOUNT SETUPS, CONTACT KRYSYAL TURNER WITH LOVE'S AT (405) 463-8959 OR BY EMAIL AT KRYSYAL.TURNER@LOVES.COM TO COORDINATE.

STANDARD DRAWINGS
THE STANDARD SPECIFICATIONS OF OHIO DEPARTMENT OF TRANSPORTATION (ODOT), CURRENT EDITION, INCLUDING STANDARD DRAWINGS SHALL GOVERN THIS IMPROVEMENT.

PROTOTYPICAL SITE EQUIPMENT LIST						
DESCRIPTION	QTY	MANUFACTURE/MODEL	PROCURED BY	INSTALLED BY	VENDOR CONTACT	REMARKS
FIRE PUMP HOUSE & STORAGE TANKS						
PUMP HOUSE PACKAGE	1	HOUSE PACKAGE 2 (1000-1250 GPM) CITY FED	VENDOR	GC	FLUID SOLUTIONS: DAVID GONZALEZ DGONZALEZ@FLUIDSLLC.COM 1-205-730-7116	

OHIO DEPARTMENT OF TRANSPORTATION STANDARD CONSTRUCTION DETAILS					
DWG NUMBER	DESCRIPTION	DWG NUMBER	DESCRIPTION	DWG NUMBER	DESCRIPTION
H.W.-2.1	HALF-HEIGHT HEADWALLS				
MGS 1.1	MIDWEST GUARDRAIL SYSTEM - RAIL COMPONENTS				
MGS 2.1	MIDWEST GUARDRAIL SYSTEM				
H.W. -1.1	FULL-HEIGHT HEADWALLS				

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



PROPERTY DATA:

PARCEL OWNER:
1
2
MGD PROPERTY HOLDINGS, LLC.
LOASE COAL INC.

PARCEL:
1
2
PARCEL ID:
39-01234.000-3
39-00338.000-31
PARCEL AREA:
7.24 ACRES
128.94 ACRES

ADDRESS:
66320 BELMONT-MORRISTOWN ROAD
ST. CLAIRSVILLE, OHIO 43718

PROPOSED USE:
TRUCK STOP

FLOODPLAIN DESIGNATION:
THE SUBJECT PARCEL IS LOCATED WITHIN "ZONE X" (AREA DETERMINED TO BE OUTSIDE OF THE 0.2% ANNUAL CHANCE FLOODPLAIN) AS INDICATED BY THE FLOOD INSURANCE RATE MAP (FIRM) COMMUNITY-PANEL NUMBER 39013C0175E, EFFECTIVE DATE: APRIL 5, 2006; PUBLISHED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY.

BENCHMARKS	
TBM "A"	"X" CHISELED IN SOUTH SIDE OF RIM OF SANITARY MANHOLE ON NORTH SIDE OF RECO DRIVE, EAST OF ADDRESS 41245 RECO DRIVE = 1239.44'
TBM "B"	EAST BOLT ON HYDRANT ON SOUTH SIDE OF RECO DRIVE, ACROSS FROM ADDRESS 41371 RECO DRIVE = 120.05'
TBM "C"	SOUTHWEST BOLT ON HYDRANT ON EAST SIDE OF BELMONT MORRISTOWN ROAD AT ADDRESS 66377 BELMONT MORRISTOWN ROAD = 1193.84'

VERTICAL DATUM: NAVD88

175 Montrose West Ave., Suite 400
Akron, OH 44321
Phone: 330.665.0660 Fax: 888.208.4826

LOVE'S TRAVEL STOP
ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

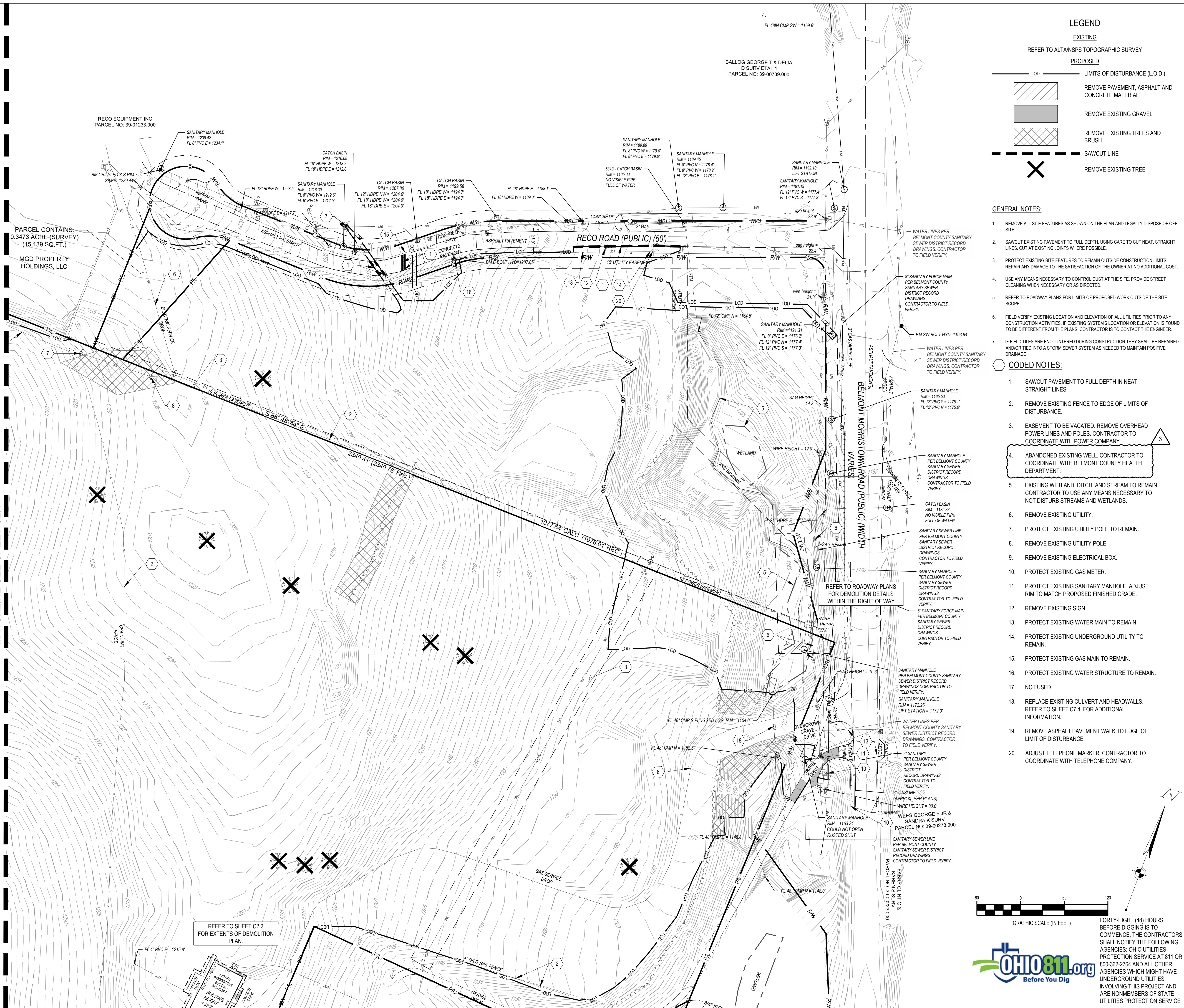
Revisions / Submissions		
ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024
5	ADDENDUM 5	04/18/2024

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Project Number: 759267
Scale: N/A
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

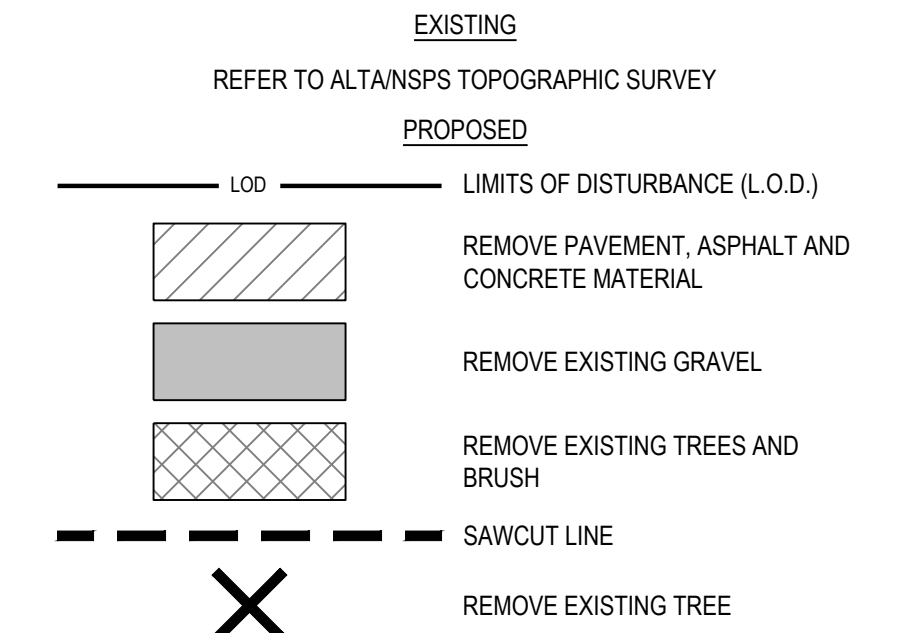
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COVER SHEET

C1.0

MATCHLINE SEE SHEET C2.1



LEGEND

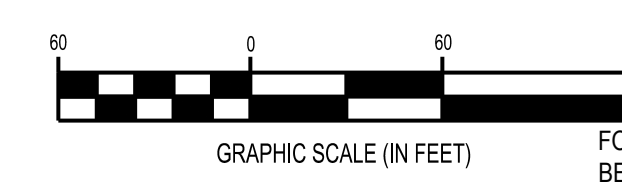


GENERAL NOTES:

- REMOVE ALL SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
- SAWCUT EXISTING PAVEMENT TO FULL DEPTH, USING CARE TO CUT NEAT, STRAIGHT LINES. CUT AT EXISTING JOINTS WHERE POSSIBLE.
- PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
- USE ANY MEANS NECESSARY TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
- REFER TO ROADWAY PLANS FOR LIMITS OF PROPOSED WORK OUTSIDE THE SITE SCOPE.
- FIELD VERIFY EXISTING LOCATION AND ELEVATION OF ALL UTILITIES PRIOR TO ANY CONSTRUCTION ACTIVITIES. IF EXISTING SYSTEMS LOCATION OR ELEVATION IS FOUND TO BE DIFFERENT FROM THE PLANS, CONTRACTOR IS TO CONTACT THE ENGINEER.
- IF FIELD TILES ARE ENCOUNTERED DURING CONSTRUCTION THEY SHALL BE REPAIRED AND/OR TIED INTO A STORM SEWER SYSTEM AS NEEDED TO MAINTAIN POSITIVE DRAINAGE.

CODED NOTES:

- SAWCUT PAVEMENT TO FULL DEPTH IN NEAT, STRAIGHT LINES
- REMOVE EXISTING FENCE TO EDGE OF LIMITS OF DISTURBANCE.
- EASEMENT TO BE VACATED. REMOVE OVERHEAD POWER LINES AND POLES. CONTRACTOR TO COORDINATE WITH POWER COMPANY
- ABANDONED EXISTING WELL. CONTRACTOR TO COORDINATE WITH BELMONT COUNTY HEALTH DEPARTMENT.
- EXISTING WETLAND, DITCH, AND STREAM TO REMAIN. CONTRACTOR TO USE ANY MEANS NECESSARY TO NOT DISTURB STREAMS AND WETLANDS.
- REMOVE EXISTING UTILITY.
- PROTECT EXISTING UTILITY POLE TO REMAIN.
- REMOVE EXISTING UTILITY POLE.
- REMOVE EXISTING ELECTRICAL BOX.
- PROTECT EXISTING GAS METER.
- PROTECT EXISTING SANITARY MANHOLE. ADJUST RIM TO MATCH PROPOSED FINISHED GRADE.
- REMOVE EXISTING SIGN.
- PROTECT EXISTING WATER MAIN TO REMAIN.
- PROTECT EXISTING UNDERGROUND UTILITY TO REMAIN.
- PROTECT EXISTING GAS MAIN TO REMAIN.
- PROTECT EXISTING WATER STRUCTURE TO REMAIN.
- NOT USED.
- REPLACE EXISTING CULVERT AND HEADWALLS. REFER TO SHEET C7.4 FOR ADDITIONAL INFORMATION.
- REMOVE ASPHALT PAVEMENT WALK TO EDGE OF LIMIT OF DISTURBANCE.
- ADJUST TELEPHONE MARKER. CONTRACTOR TO COORDINATE WITH TELEPHONE COMPANY.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

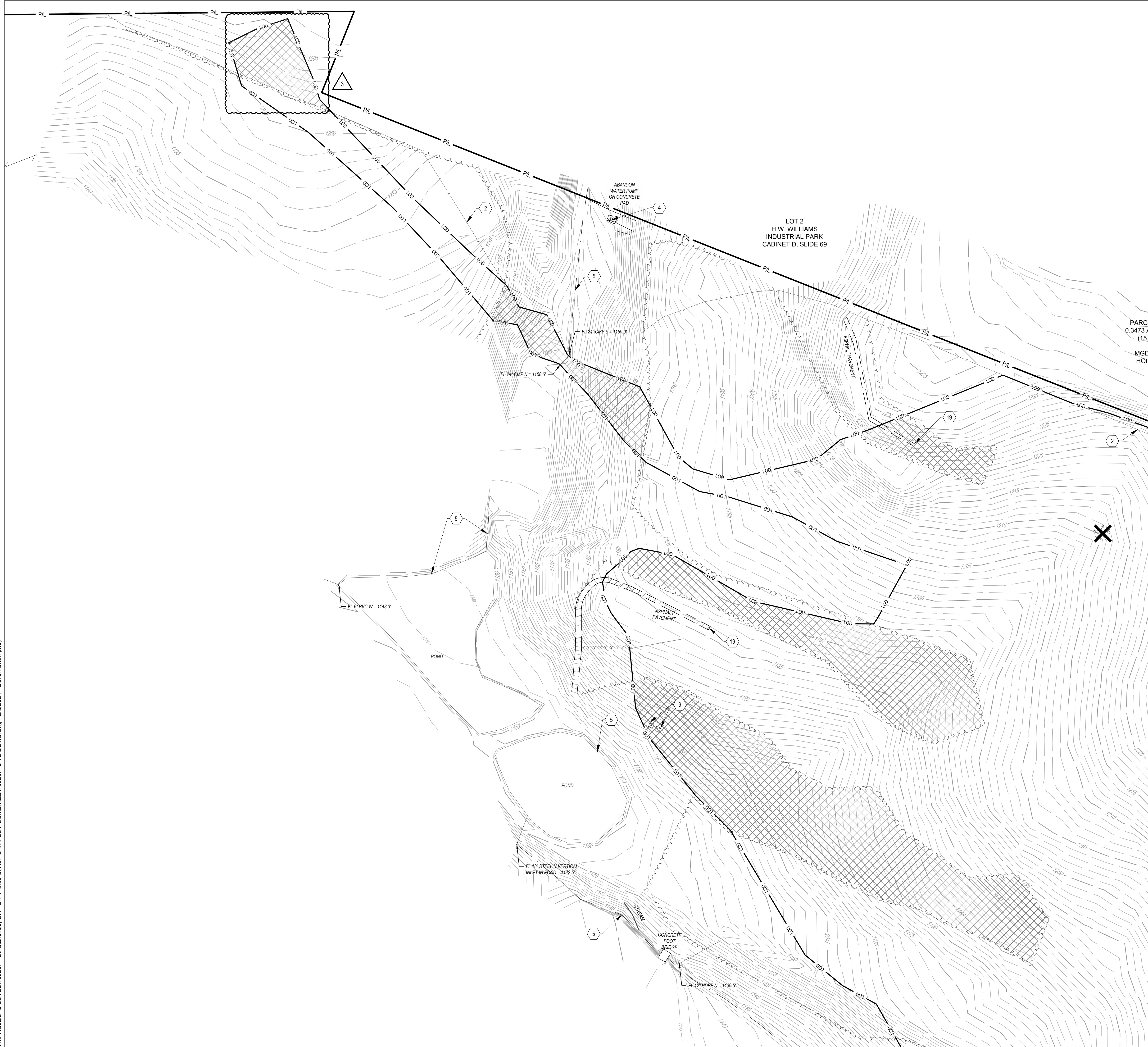
Revisions / Submissions		
ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024

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Project Number: 759267
Scale: 1" = 60'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
DEMOLITION PLAN - EAST

C2.0

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE DEMO.dwg - 2/8/2024 - Devon Champney



MATCHLINE SEE SHEET C2.0

LEGEND

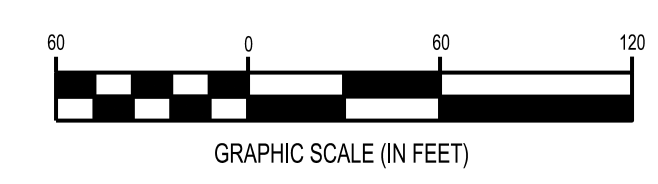
- REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- EXISTING**
- PROPOSED**
- L.O.D. — LIMITS OF DISTURBANCE (L.O.D.)
 - REMOVE PAVEMENT, ASPHALT AND CONCRETE MATERIAL
 - REMOVE EXISTING GRAVEL
 - REMOVE EXISTING TREES AND BRUSH
 - - - SAWCUT LINE
 - REMOVE EXISTING TREE

GENERAL NOTES:

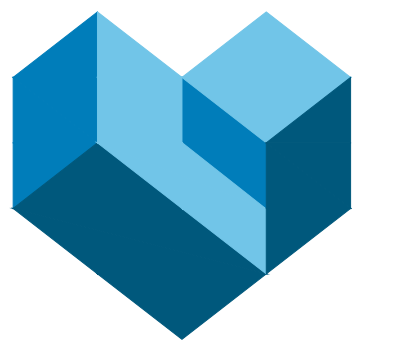
1. REMOVE ALL SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
2. SAWCUT EXISTING PAVEMENT TO FULL DEPTH, USING CARE TO CUT NEAT, STRAIGHT LINES. CUT AT EXISTING JOINTS WHERE POSSIBLE.
3. PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
4. USE ANY MEANS NECESSARY TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
5. REFER TO ROADWAY PLANS FOR LIMITS OF PROPOSED WORK OUTSIDE THE SITE SCOPE.
6. FIELD VERIFY EXISTING LOCATION AND ELEVATION OF ALL UTILITIES PRIOR TO ANY CONSTRUCTION ACTIVITIES. IF EXISTING SYSTEMS LOCATION OR ELEVATION IS FOUND TO BE DIFFERENT FROM THE PLANS, CONTRACTOR IS TO CONTACT THE ENGINEER.
7. IF FIELD TILES ARE ENCOUNTERED DURING CONSTRUCTION THEY SHALL BE REPAIRED AND/OR TIED INTO A STORM SEWER SYSTEM AS NEEDED TO MAINTAIN POSITIVE DRAINAGE.

CODED NOTES:

1. SAWCUT PAVEMENT TO FULL DEPTH IN NEAT, STRAIGHT LINES
2. REMOVE EXISTING FENCE TO EDGE OF LIMITS OF DISTURBANCE.
3. EASEMENT TO BE VACATED. REMOVE OVERHEAD POWER LINES AND POLES. CONTRACTOR TO COORDINATE WITH POWER COMPANY.
4. ABANDONED EXISTING WELL. CONTRACTOR TO COORDINATE WITH BELMONT COUNTY HEALTH DEPARTMENT.
5. EXISTING WETLAND, DITCH, AND STREAM TO REMAIN. CONTRACTOR TO USE ANY MEANS NECESSARY TO NOT DISTURB STREAMS AND WETLANDS.
6. REMOVE EXISTING UTILITY.
7. PROTECT EXISTING UTILITY POLE TO REMAIN.
8. REMOVE EXISTING UTILITY POLE.
9. REMOVE EXISTING ELECTRICAL BOX.
10. PROTECT EXISTING GAS METER.
11. PROTECT EXISTING SANITARY MANHOLE. ADJUST RIM TO MATCH PROPOSED FINISHED GRADE.
12. REMOVE EXISTING SIGN.
13. PROTECT EXISTING WATER MAIN TO REMAIN.
14. PROTECT EXISTING UNDERGROUND UTILITY TO REMAIN.
15. PROTECT EXISTING GAS MAIN TO REMAIN.
16. PROTECT EXISTING WATER STRUCTURE TO REMAIN.
17. NOT USED.
18. REPLACE EXISTING CULVERT AND HEADWALLS. REFER TO SHEET C7.4 FOR ADDITIONAL INFORMATION.
19. REMOVE ASPHALT PAVEMENT WALK TO EDGE OF LIMIT OF DISTURBANCE.
20. ADJUST TELEPHONE MARKER. CONTRACTOR TO COORDINATE WITH TELEPHONE COMPANY.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



CESO
WWW.CESOINC.COM

175 Monrovia West Ave., Suite 400
Alvord, OH 44321
Phone: 330.665.0660 Fax: 888.208.4826



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

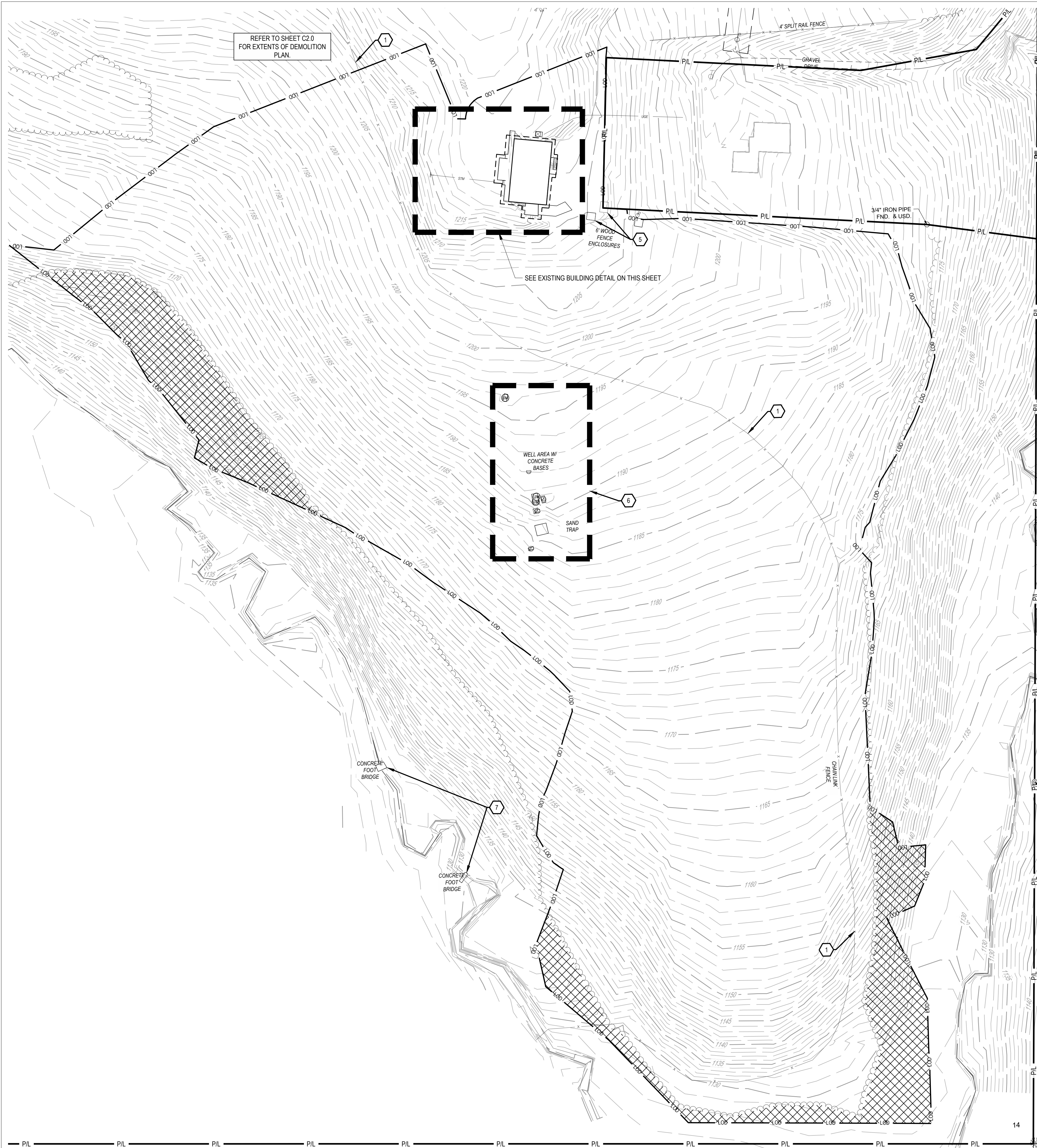
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
ID	Description	Date
3	ADDENDUM 3	02/09/2024

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 Project Number: 759267
 Scale: 1" = 60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
DEMOLITION PLAN - WEST

C2.1



LEGEND

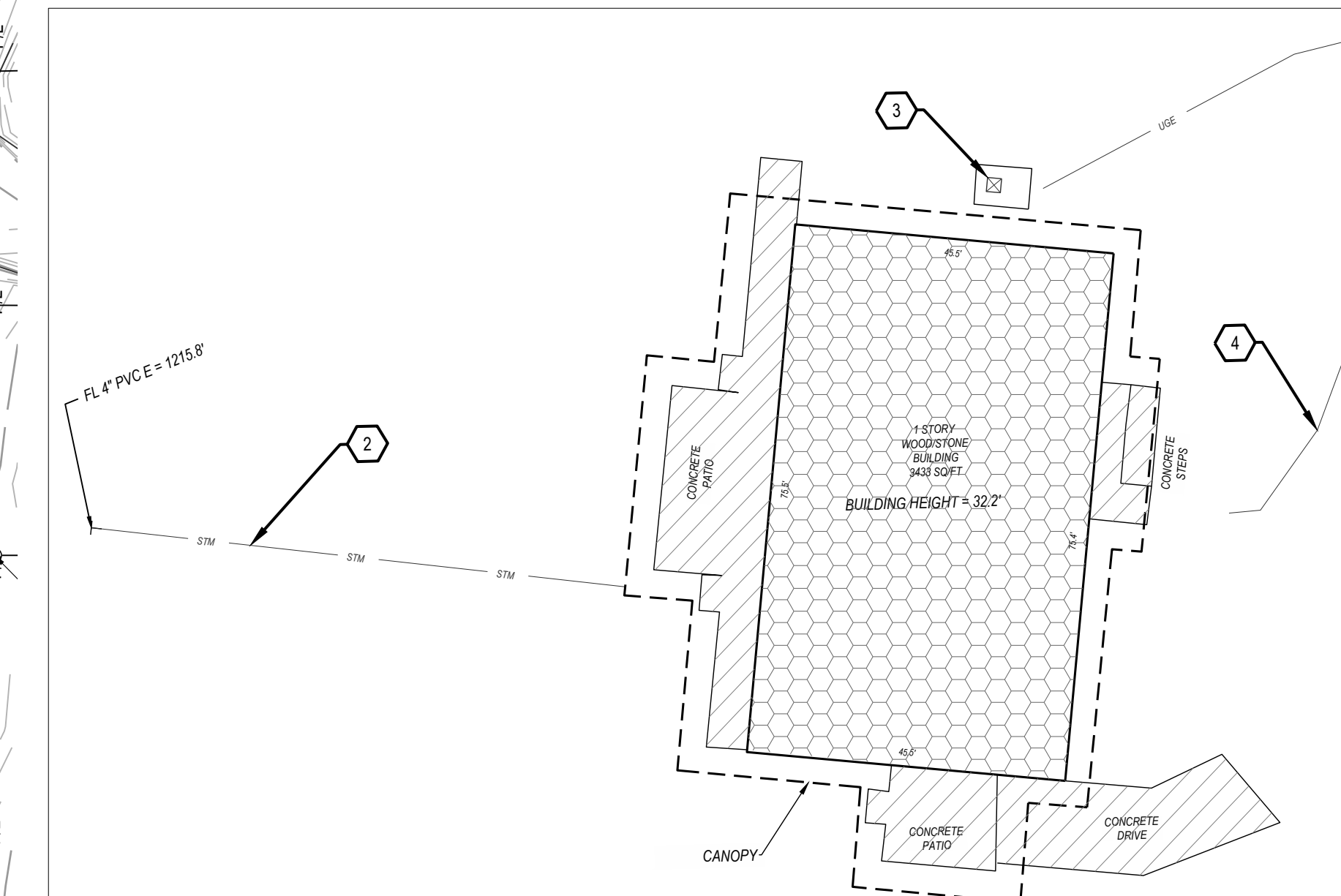
- EXISTING**
REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- LOD LIMITS OF DISTURBANCE (L.O.D.)
 - [Grid Pattern] REMOVE EXISTING STRUCTURE, INCLUDING FOOTERS
 - [Cross-hatch Pattern] REMOVE PAVEMENT AND CONCRETE MATERIAL
 - [Cross-hatch Pattern] REMOVE EXISTING TREES AND BRUSH

GENERAL NOTES:

1. REMOVE ALL SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
2. SAWCUT EXISTING PAVEMENT TO FULL DEPTH, USING CARE TO CUT NEAT, STRAIGHT LINES. CUT AT EXISTING JOINTS WHERE POSSIBLE.
3. PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
4. USE ANY MEANS NECESSARY TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
5. REFER TO ROADWAY PLANS FOR LIMITS OF PROPOSED WORK OUTSIDE THE SITE SCOPE.
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7. IF FIELD TILES ARE ENCOUNTERED DURING CONSTRUCTION THEY SHALL BE REPAIRED AND/OR TIED INTO A STORM SEWER SYSTEM AS NEEDED TO MAINTAIN POSITIVE DRAINAGE.

CODED NOTES:

1. REMOVE EXISTING FENCE
2. REMOVE EXISTING STORM SEWER
3. REMOVE EXISTING ELECTRICAL STRUCTURE. COORDINATE WITH ELECTRIC COMPANY
4. REMOVE EXISTING GAS CONNECTION. COORDINATE WITH GAS COMPANY
5. REMOVE WOODEN FENCE ENCLOSURES
6. REMOVE EXISTING STRUCTURES AND PIPES
7. EXISTING STRUCTURE TO REMAIN

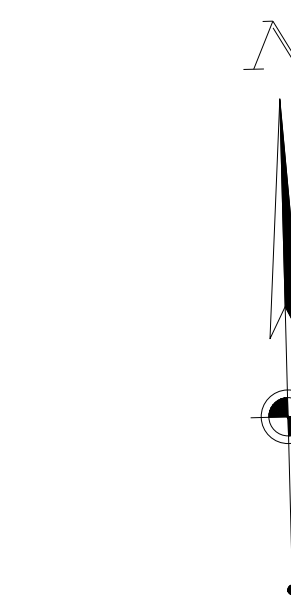


EXISTING BUILDING DETAIL

SCALE: 1" = 20'



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2784 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



CESO
WWW.CESOINC.COM

175 Monrovia West Ave., Suite 400
Akron, OH 44321
Phone: 330.665.0660 Fax: 888.208.4826



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

ID	Description	Date
3	ADDENDUM 3	02/09/2024

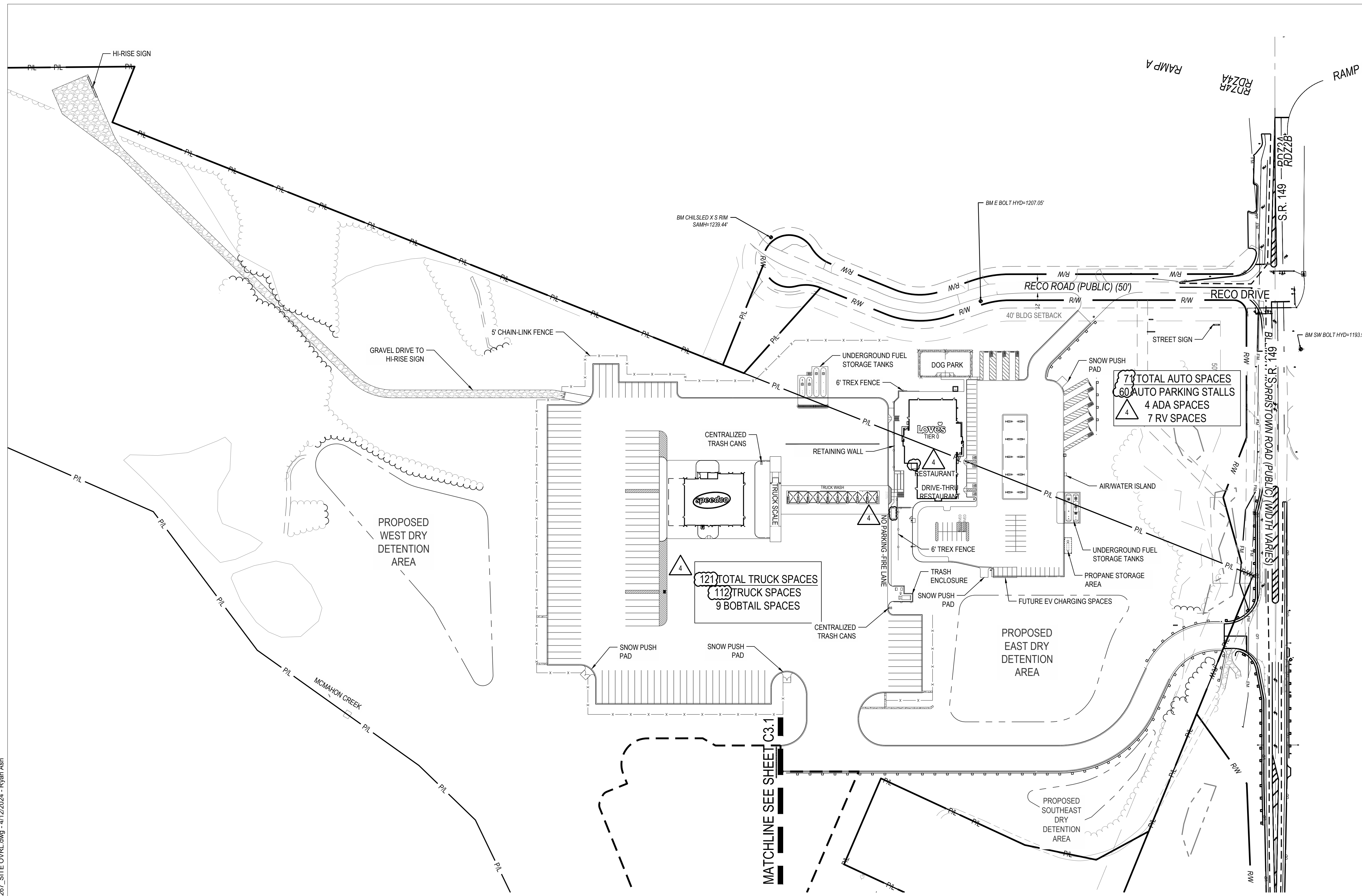
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Project Number: 759267
Scale: 1" = 60'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
DEMOLITION PLAN - PREPARED PAD AREA

C2.2

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLANS\LOT-COMBINED\759267 SITE OVR.dwg - 4/12/2024 - Ryan Ash



LEGEND

EXISTING
REFER TO ALTA/SPS FOR EXISTING FEATURES LEGEND

PROPOSED

- CONCRETE CURB AND GUTTER
- PAVEMENT
- 6' TREX FENCE (REF. ARCH. PLANS)
- 5' CHAIN-LINK FENCE (REF. ARCH. PLANS)
- 4' BLACK CHAIN-LINK FENCE (REF. ARCH. PLANS)
- UTILITY EASEMENT

LIGHT POLE SHOWN FOR REFERENCE ONLY. REFER TO ARCH. PLANS FOR ADDITIONAL INFORMATION AND EXACT LOCATION

SITE REQUIREMENTS:

BUILDING SETBACKS:	FRONT YARD - 40', REAR YARD - 0' RIGHT YARD - 50', LEFT YARD - 0'
PARKING SETBACKS:	NONE
STANDARD PARKING DIMENSIONS:	CAR: 9'Wx18'L (PERIMETER) 10'Wx20'L (INTERIOR) TRUCK: 12'6"Wx35'L (PERIMETER) 12'6"Wx75'L (INTERIOR) RV: 10'Wx40'L
MINIMUM DRIVE AISLE:	NONE

SITE ANALYSIS:

PROPOSED BUILDING AREA	LOVE'S COUNTRY STORE: 12,393 ± S.F. ATTACHED RESTAURANT: 2,998 ± S.F. DRIVE-THRU RESTAURANT: 3,221 ± S.F. TIRE SHOP: 11,135 ± S.F.
TOTAL:	29,747 ± S.F.

PARKING PROVIDED:

PARKING DIMENSIONS: VARIES

- 4 ADA CAR PARKING SPACES
- 60 STANDARD CAR PARKING SPACES
- 7 RV PARKING SPACES
- 9 BOBTAIL PARKING SPACES
- 112 TRUCK PARKING SPACES
- 192 TOTAL PARKING SPACES

LOVE'S TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

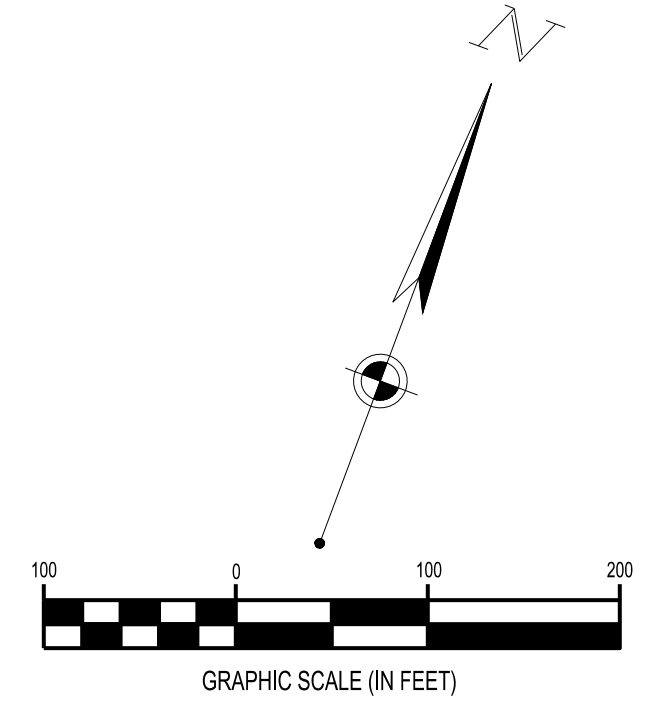
Revisions / Submissions

ID	Description	Date
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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Project Number:	759267
Scale:	1" = 100'
Drawn By:	FAR
Checked By:	JTK
Date:	12/19/2023
Issue:	OUT TO BID

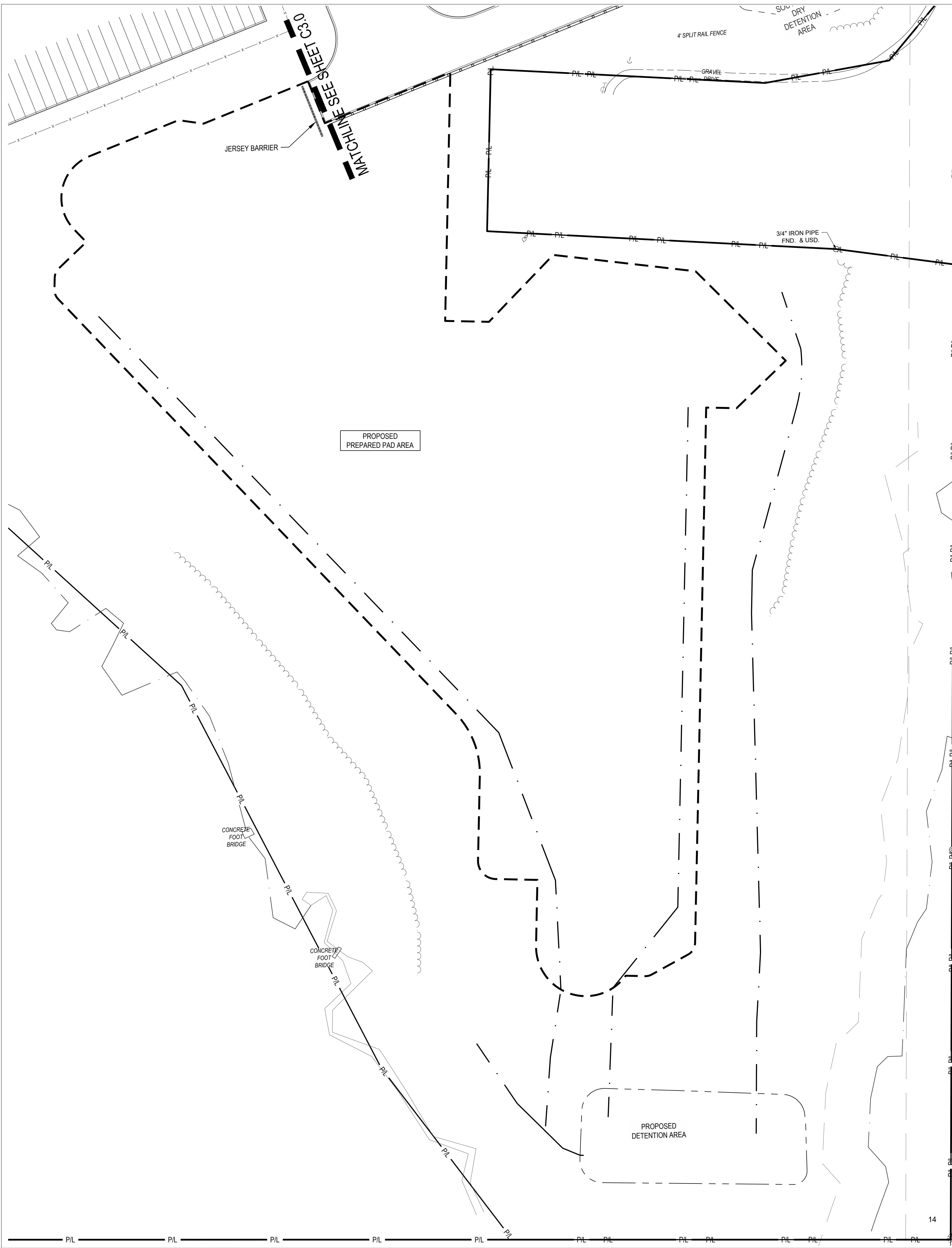
Drawing Title:
OVERALL SITE PLAN



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

C3.0

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267_RV SITE OVRL.dwg - 12/19/2023 - Francesca Rapposelli



LEGEND
 EXISTING
 REFER TO ALTANSPS FOR EXISTING FEATURES LEGEND
 SWALE



CESO
 WWW.CESOINC.COM

175 Monrovia West Ave., Suite 400
 Akron, OH 44321
 Phone: 330.665.0660 Fax: 888.208.4826



LOVES TRAVEL STOP
ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
ID	Description	Date

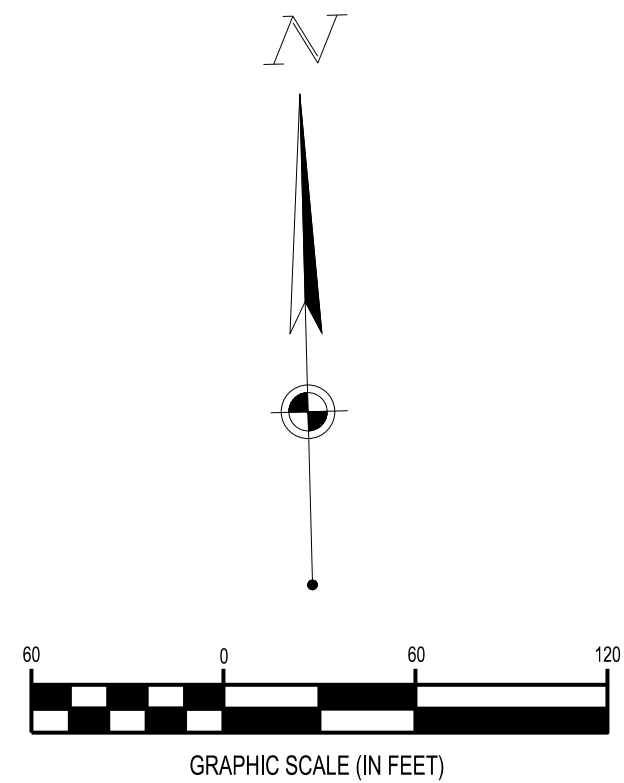
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 Project Number: 759267
 Scale: 1" = 60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
OVERALL SITE PLAN - PREPARED PAD AREA

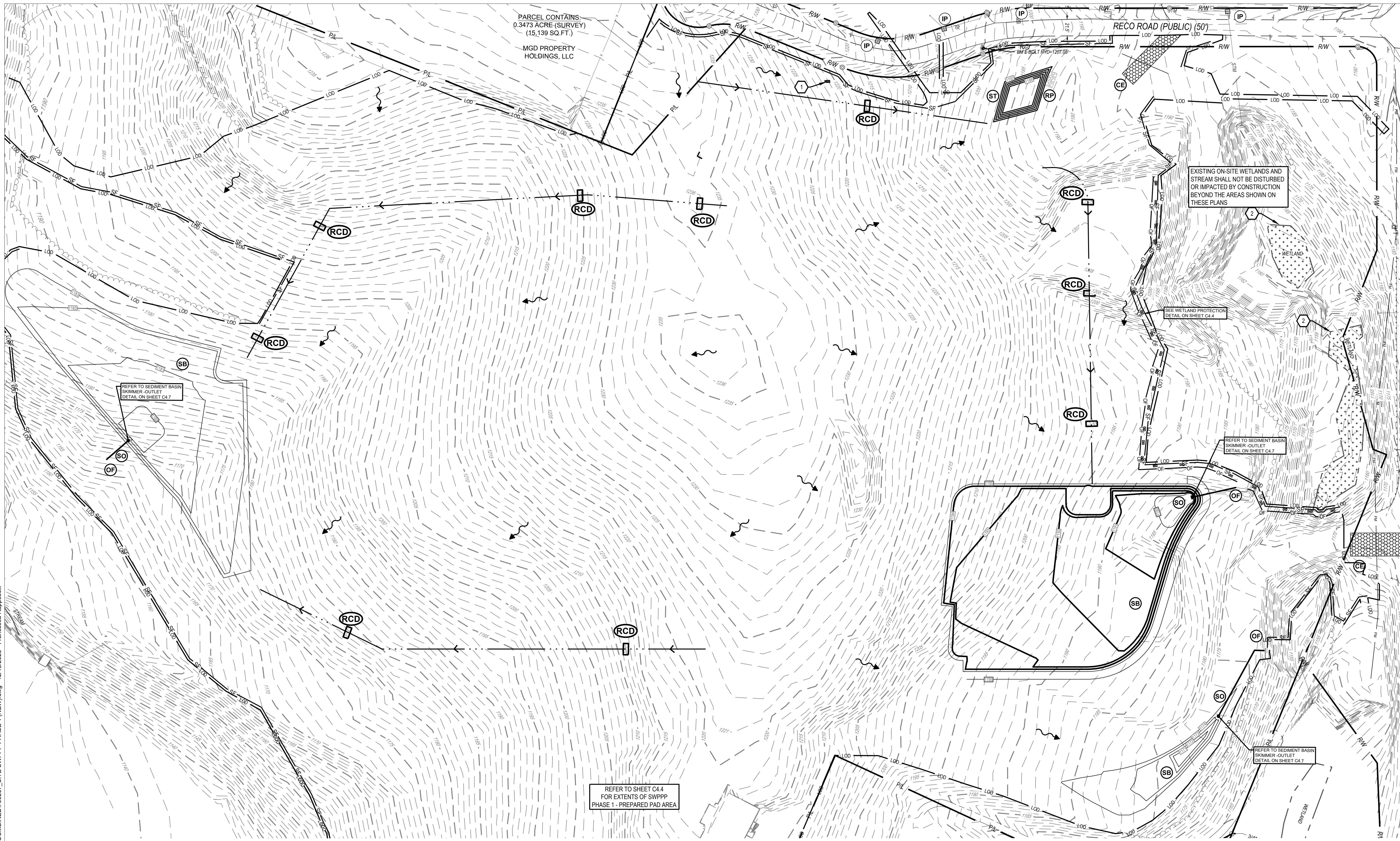
C3.1



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\OT-COMBINED\759267 SITE SWPPP PHASE 1 (NEW).dwg - 12/18/2023 - Francesca Rapposelli



GENERAL NOTES

- ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED AS IDENTIFIED WITH OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) AND LOCAL JURISDICTION INSPECTOR.
- CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION, IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
- ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA).
- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- REMOVE ALL ON SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
- PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
- USE ANY MEANS NECESSARY AND ACCEPTABLE TO THE JURISDICTION TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
- SILT FENCE SHOWN OFF OF LIMITS OF DISTURBANCE FOR CLARITY PURPOSES ONLY. CONTRACTOR TO ENSURE SILT FENCE IS PLACED AT LIMITS OF DISTURBANCE. ANY DISCREPANCIES SHOULD BE BROUGHT TO THE ENGINEER PRIOR TO PLACEMENT OF ANY EROSION CONTROL MEASURES.
- TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 6' BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS, OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.

SEQUENCE OF CONSTRUCTION - (PHASE ONE)

- UNLESS NOTED OTHERWISE, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL MEASURES IN STRICT ACCORDANCE WITH THE SWPPP THROUGHOUT THE DURATION OF THE PROJECT.
- ENSURE NOTICE OF INTENT (N.O.I.) IS FILED. KEEP A COPY OF THE PERMIT AND SWPPP ON SITE.
 - INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SWPPP INCLUDING CULVERT UNDER ACCESS DRIVE FROM RECO ROAD.
 - INSPECTION OF EROSION CONTROL MEASURES AS OUTLINED IN NOTES, REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
 - SITE DEMOLITION AND CLEARING.

NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION.

LEGEND

REFER TO ALTA/NSPS TOPOGRAPHIC SURVEY

EXISTING

- SF — SF — PAVEMENT
- - - - - SILT FENCE
- - - - - RIDGE LINE
- L.O.D. --- GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
- → → → → TEMPORARY DIVERSION CHANNEL
- ↗ FLOW ARROW
- ⊙ TREULOOK CAMERA
- ⊙ RCD ⊙ ROCK CHECK DAM
- ⊙ ST ⊙ TEMPORARY SEDIMENT TRAP
- ⊙ SB ⊙ TEMPORARY SEDIMENT BASIN
- ⊙ OF ⊙ STORMWATER OUTFALL POINT

PROPOSED

- ⊙ SO ⊙ SKIMMER OUTLET STRUCTURE
- ⊙ CE ⊙ STABILIZED CONSTRUCTION ENTRANCE
- ⊙ CW ⊙ CONCRETE WASHOUT
- ⊙ RP ⊙ RIP RAP STORM SEWER OUTLET/SLOPE PROTECTION
- ⊙ ⊙ EXISTING WETLAND

REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
 REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN

CODED NOTES

- TRUELOOK CAMERA.
- WETLAND STREAM AREAS TO BE PROTECTED THROUGHOUT CONSTRUCTION.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

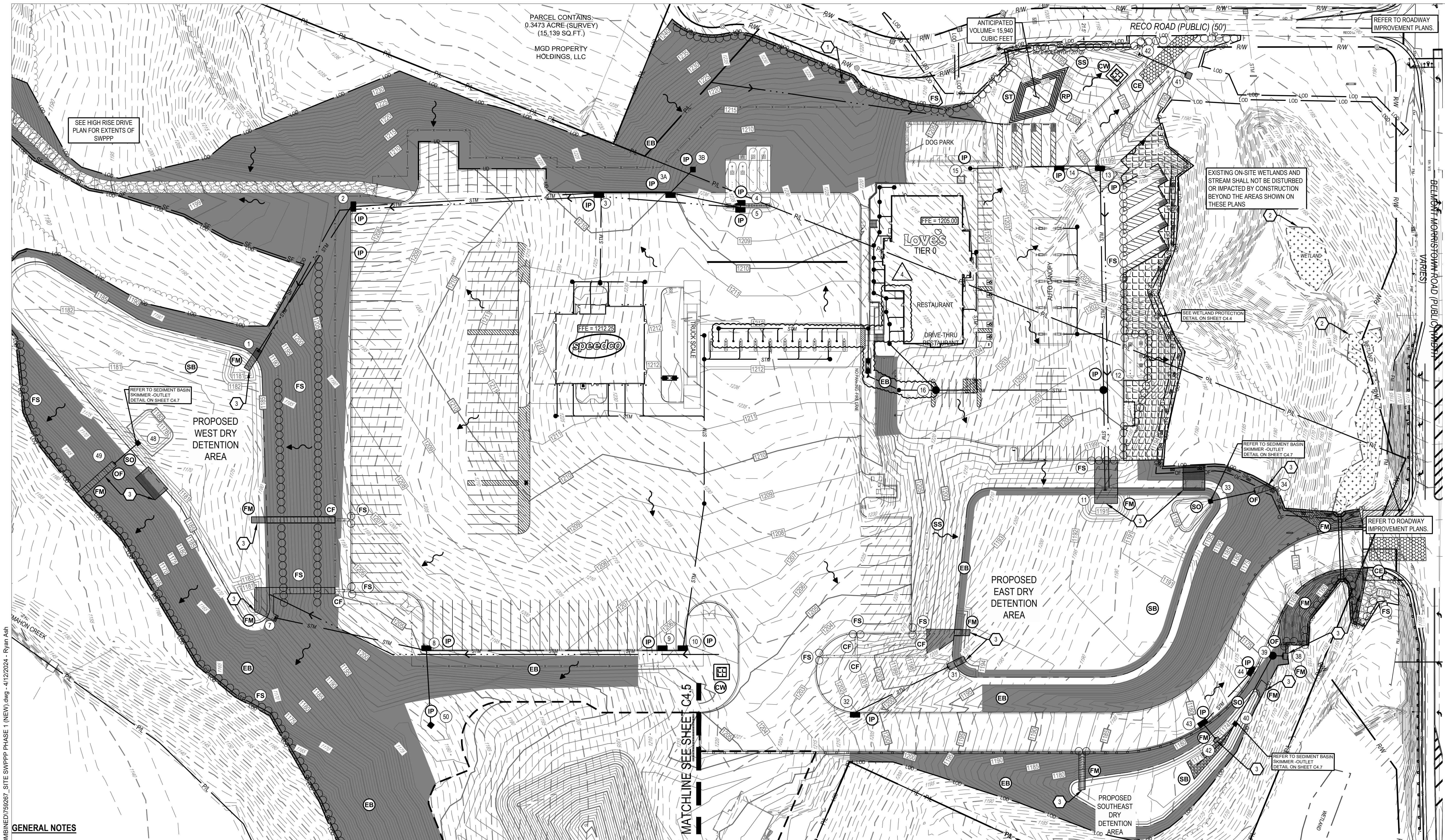
ID	Description	Date

Drawing Title: **SWPPP PHASE 1**

Project Number: 759267
 Scale: 1" = 60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

C4.1

W:\PROJECTS\LOVES\75927 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\75927 SITE SWPPP PHASE 1 (NEW).dwg - 4/12/2024 - Ryan Ash



GENERAL NOTES

- ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED AS IDENTIFIED WITH OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) AND LOCAL JURISDICTION INSPECTOR.
- CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION, IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
- ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA)
- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- REMOVE ALL ON SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
- PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
- USE ANY MEANS NECESSARY AND ACCEPTABLE TO THE JURISDICTION TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
- SILT FENCE SHOWN OFF OF LIMITS OF DISTURBANCE FOR CLARITY PURPOSES ONLY. CONTRACTOR TO ENSURE SILT FENCE IS PLACED AT LIMITS OF DISTURBANCE. ANY DISCREPANCIES SHOULD BE BROUGHT TO THE ENGINEER PRIOR TO PLACEMENT OF ANY EROSION CONTROL MEASURES.
- TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 9' BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS, OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.

SEQUENCE OF CONSTRUCTION - (PHASE TWO)

- UNLESS NOTED OTHERWISE, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL MEASURES IN STRICT ACCORDANCE WITH THE SWPPP THROUGHOUT THE DURATION OF THE PROJECT.
- INSPECTION OF EROSION CONTROL MEASURES INSTALLED UNDER PHASE ONE AS OUTLINED IN NOTES. REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
 - INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SOIL EROSION PLAN FOR PHASE TWO.
 - ENSURE POSITIVE DRAINAGE FROM SITE TO BASIN DURING ALL PHASES OF CONSTRUCTION. STORM DITCHES OR BERMS MAY BE NECESSARY PRIOR TO STORM STRUCTURE CONSTRUCTION.
 - ROUGH GRADING. PROVIDE SOIL STABILIZATION OF DISTURBED AREAS WHICH ARE INACTIVE. REFER TO NPDES PERMIT REQUIREMENTS AND SWPPP DETAILS FOR ADDITIONAL INFORMATION.
 - STORM SEWER SYSTEM AND UNDERGROUND UTILITY CONSTRUCTION. INSTALL INLET PROTECTION CONCURRENT WITH CONSTRUCTION OF PROPOSED STORM SEWER STRUCTURES.
 - BUILDING PAD CONSTRUCTION.
 - SEDIMENT TRAP REMOVAL.
 - CURB CONSTRUCTION.
 - FINE GRADING AND PAVEMENT SUBGRADE PREPARATION.
 - ASPHALT PAVING AND REMAINING CONCRETE FLATWORK.
 - PREPARATION OF LANDSCAPED AREAS TO FINISHED GRADE.

CODED NOTES

- TRUELOOK CAMERA.
- WETLAND STREAM AREAS TO BE PROTECTED THROUGHOUT CONSTRUCTION.
- CONTRACTOR TO INSTALL EROSION CONTROL MEASURE FLEXAMAT OR APPROVED EQUAL.

LEGEND

REFER TO AT&N'S TOPOGRAPHIC SURVEY

EXISTING

- 080 MAJOR CONTOUR
- 081 MINOR CONTOUR
- BUILDING
- CONCRETE CURB AND GUTTER
- PAVEMENT
- STM STORM SEWER
- SF SILT FENCE
- RIGE LINE
- TEMPORARY DIVERSION CHANNEL
- GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
- FLOW ARROW
- TRUELOOK CAMERA

PROPOSED

- IP INLET PROTECTION
- RP RIP RAP STORM SEWER OUTLET/SLOPE PROTECTION SKIMMER OUTLET STRUCTURE
- SO ROCK CHECK DAM
- SS SOIL STABILIZATION
- FS FILTER SOCK
- ST TEMPORARY SEDIMENT TRAP
- SB TEMPORARY SEDIMENT BASIN
- OF STORMWATER OUTFALL POINT
- CF CONCRETE FLUME
- CE STABILIZED CONSTRUCTION ENTRANCE
- FM STANDARD FLEXAMAT
- CW CONCRETE WASHOUT
- EB EROSION CONTROL BLANKET
- EXISTING WETLAND
- CONTRACTOR TO PROVIDE TEMPORARY SOIL STABILIZATION UNTIL SOD IS PLACED

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Before You Dig

GRAPHIC SCALE (IN FEET)

0 30 60 90 120

REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES

REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS

REFER TO SHEETS C7 SERIES FOR GRADING PLAN

NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION.

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

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Revisions / Submissions

ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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Project Number: 75927

Scale: 1" = 60'

Drawn By: FAR

Checked By: JTK

Date: 12/19/2023

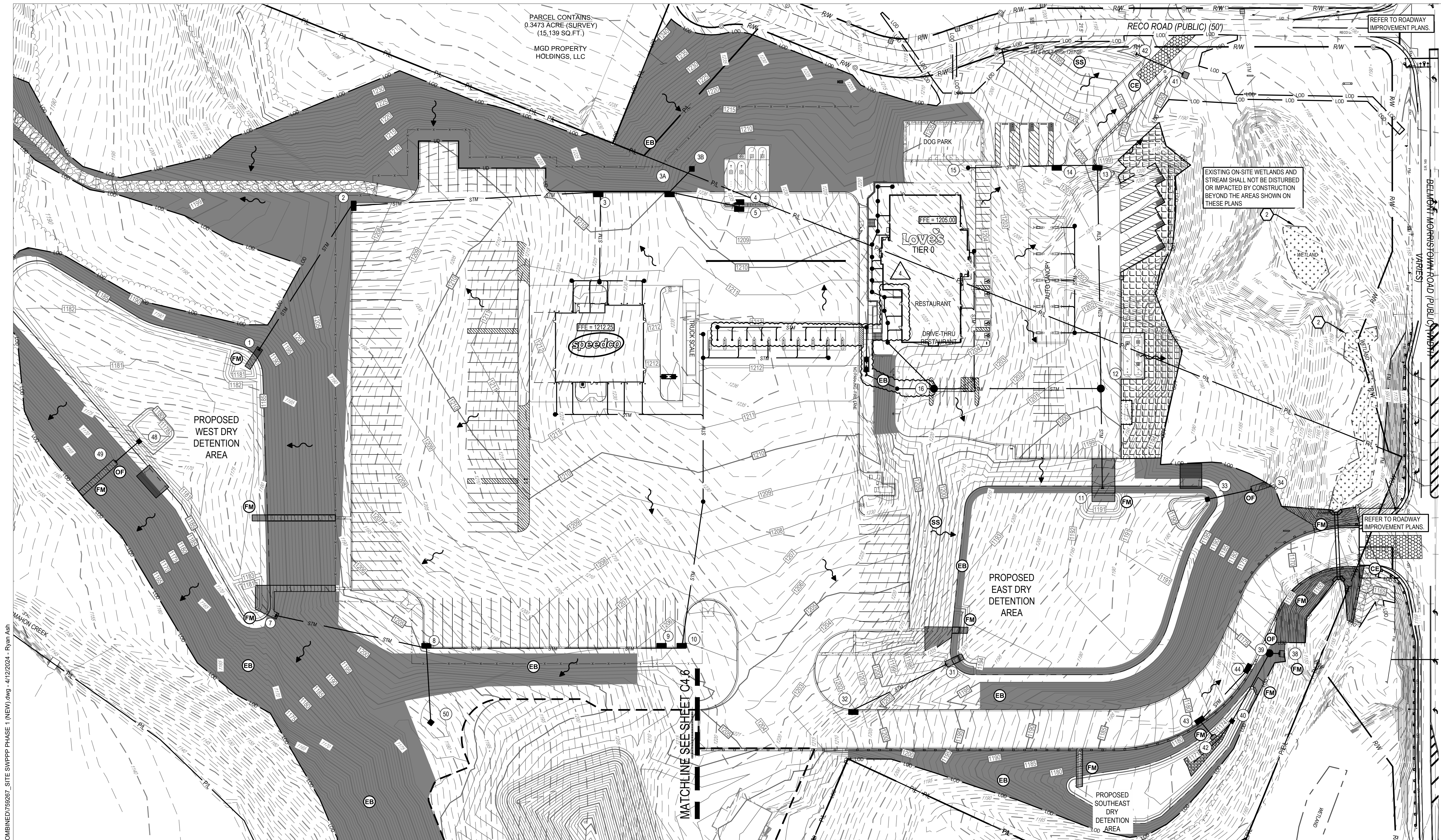
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Drawing Title:

SWPPP PHASE 2

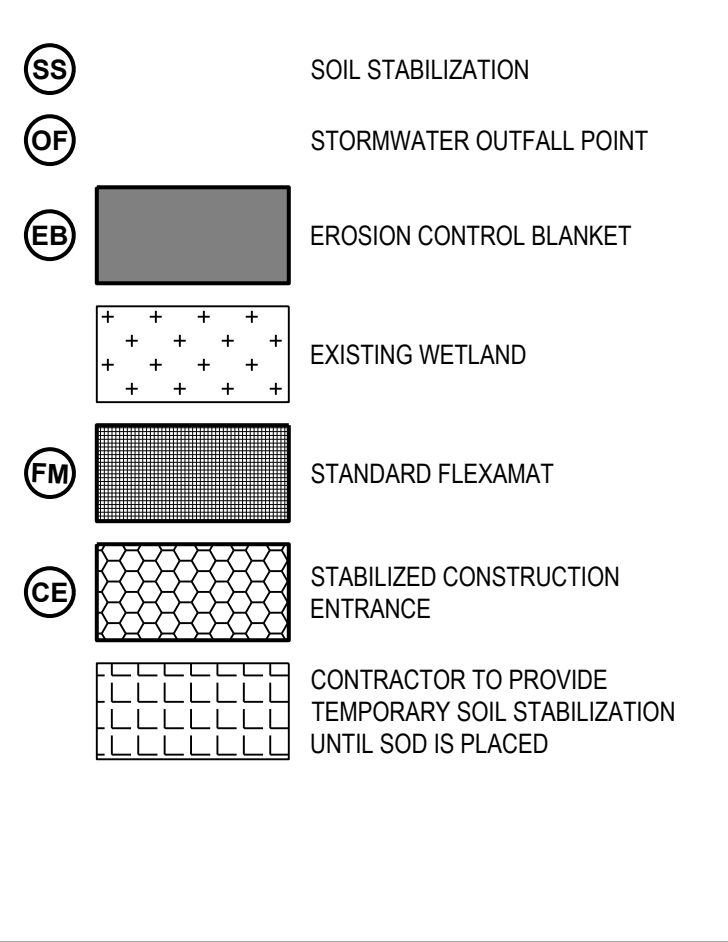
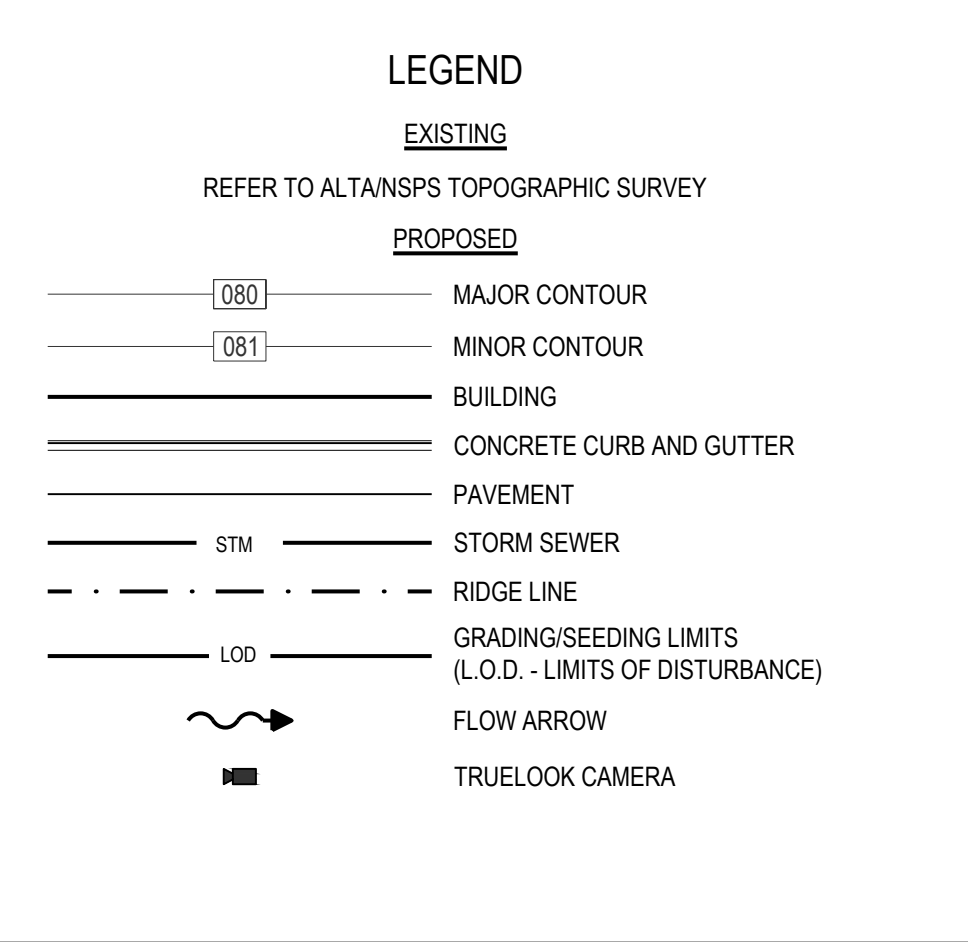
C4.2

W:\PROJECTS\LOVES\59267 - St. Clairsville, OH - SR 149\03\CIVIL\PLAN\DOT-COMBINED\759267 SITE SWPPP PHASE 3 (NEW).dwg - 4/12/2024 - Ryan Ash



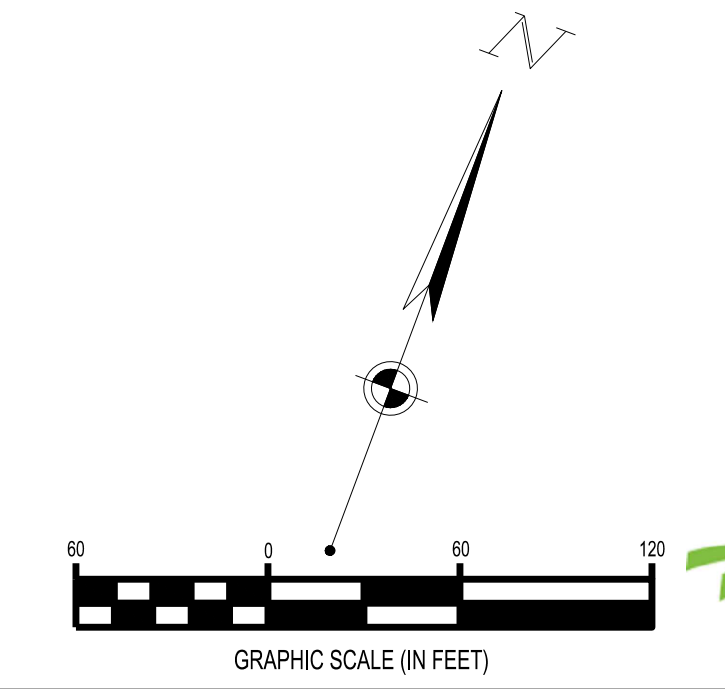
- GENERAL NOTES**
- ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED AS IDENTIFIED WITH OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) AND LOCAL JURISDICTION INSPECTOR.
 - CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION, IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
 - ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA).
 - CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
 - TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 6" BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS, OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.

- SEQUENCE OF CONSTRUCTION - (PHASE THREE)**
- INSPECTION OF EROSION CONTROL MEASURES INSTALLED UNDER PHASE ONE AND TWO AS OUTLINED IN NOTES. REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
 - INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SOIL EROSION PLAN FOR PHASE TWO.
 - REMOVE TEMPORARY RISER AND SKIMMER IN ALL SEDIMENT BASINS, AND REMOVE ACCUMULATED SEDIMENT FROM BASINS, INLET INSERTS AND STORM STRUCTURE SUMPS FOR FINAL CONVERSION TO DRY BASINS.
 - COMPLETE FINE GRADING AND PREPARATION OF LANDSCAPED AREAS TO FINISHED GRADE.
 - PERMANENT BMP'S.
 - FINAL STABILIZATION AND PERMANENT SEEDING. REFER TO THE NPDES REQUIREMENTS FOR ADDITIONAL INFORMATION. REFER TO LANDSCAPE PLAN FOR SOD LIMITS AND PLANTING AREAS.
 - REMOVE SOIL EROSION CONTROL MEASURES AFTER PERMANENT VEGETATION HAS BEEN ESTABLISHED. ENSURE NOTICE OF COMPLETION (N.O.C.) IS FILED.
- NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION. CONTRACTOR IS ALSO RESPONSIBLE FOR REMOVAL OF ALL TEMPORARY EROSION CONTROL MEASURES AFTER PERMANENT VEGETATION HAS BEEN ESTABLISHED.



REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
REFER TO SHEETS C7 SERIES FOR GRADING PLAN

- CODED NOTES**
- TRUELOOK CAMERA.
 - WETLAND STREAM AREAS TO BE PROTECTED THROUGHOUT CONSTRUCTION.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

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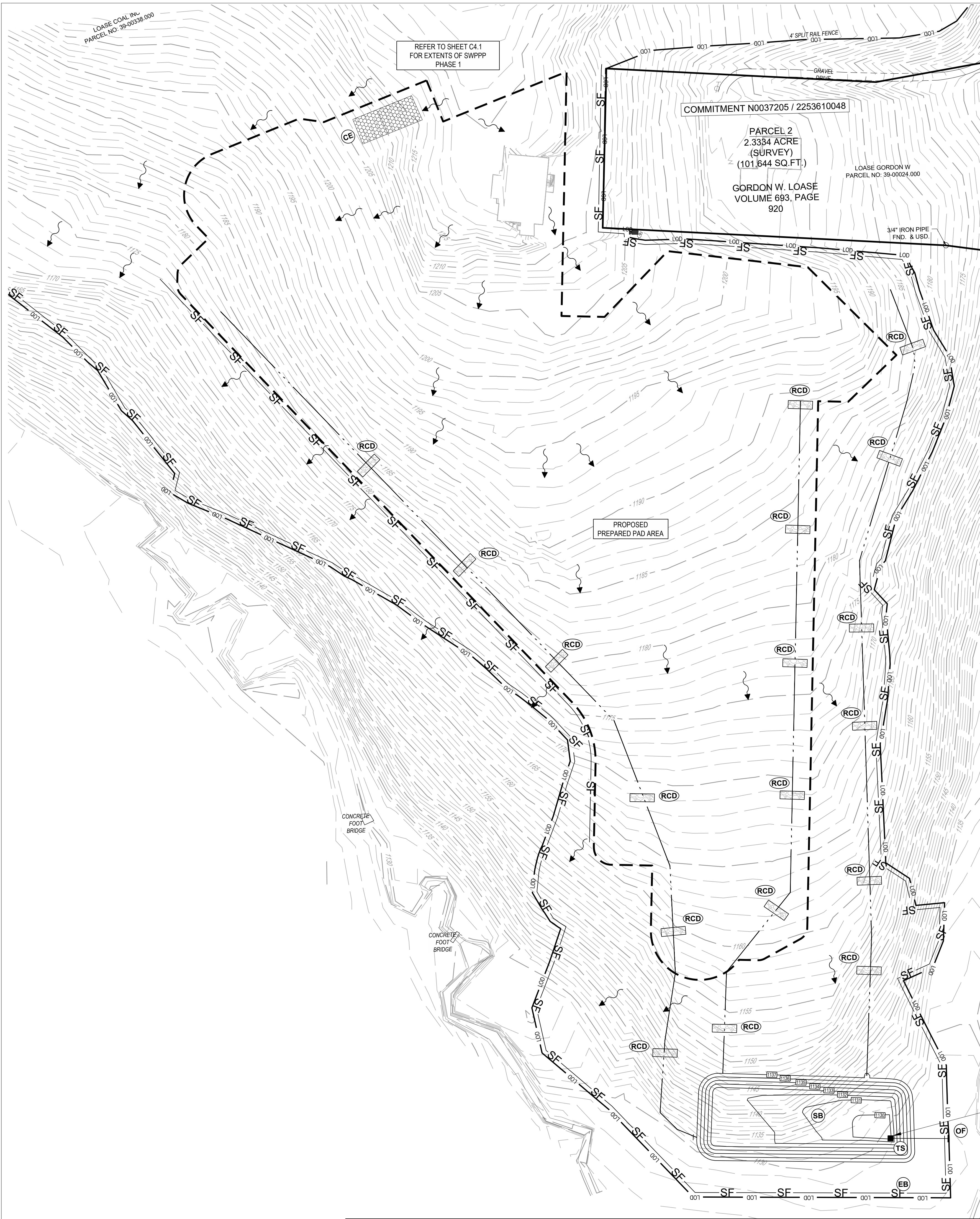
Revisions / Submissions		
ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
Scale: 1"=60'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
SWPPP PHASE 3

C4.3



GENERAL NOTES

- ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED AS IDENTIFIED WITH THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) AND LOCAL JURISDICTION INSPECTOR.
- CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION. IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
- ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH THE OEPA.
- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- REMOVE ALL ON SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
- PROTECT EXISTING SITE FEATURES TO REMAIN OUTSIDE CONSTRUCTION LIMITS. REPAIR ANY DAMAGE TO THE SATISFACTION OF THE OWNER AT NO ADDITIONAL COST.
- USE ANY MEANS NECESSARY AND ACCEPTABLE TO THE JURISDICTION TO CONTROL DUST AT THE SITE. PROVIDE STREET CLEANING WHEN NECESSARY OR AS DIRECTED.
- SILT FENCE SHOWN OFF OF LIMITS OF DISTURBANCE FOR CLARITY PURPOSES ONLY. CONTRACTOR TO ENSURE SILT FENCE IS PLACED AT LIMITS OF DISTURBANCE. ANY DISCREPANCIES SHOULD BE BROUGHT TO THE ENGINEER PRIOR TO PLACEMENT OF ANY EROSION CONTROL MEASURES.
- TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 6' BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS, OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.

LEGEND

- EXISTING**
- REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- PAVEMENT
 - SF SILT FENCE
 - RIDGE LINE
 - TEMPORARY DIVERSION SWALE
 - GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
 - FLOW ARROW
 - TRUELOOK CAMERA
 - RP RIPRAP STORM SEWER OUTLET/SLOPE PROTECTION
 - TS TEMPORARY SKIMMER
 - SB TEMPORARY SEDIMENT BASIN
 - OF STORMWATER OUTFALL POINT
 - RCD ROCK CHECK DAM

REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
 REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN

SEQUENCE OF CONSTRUCTION - (PHASE ONE)

UNLESS NOTED OTHERWISE, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL MEASURES IN STRICT ACCORDANCE WITH THE SWPPP THROUGHOUT THE DURATION OF THE PROJECT.

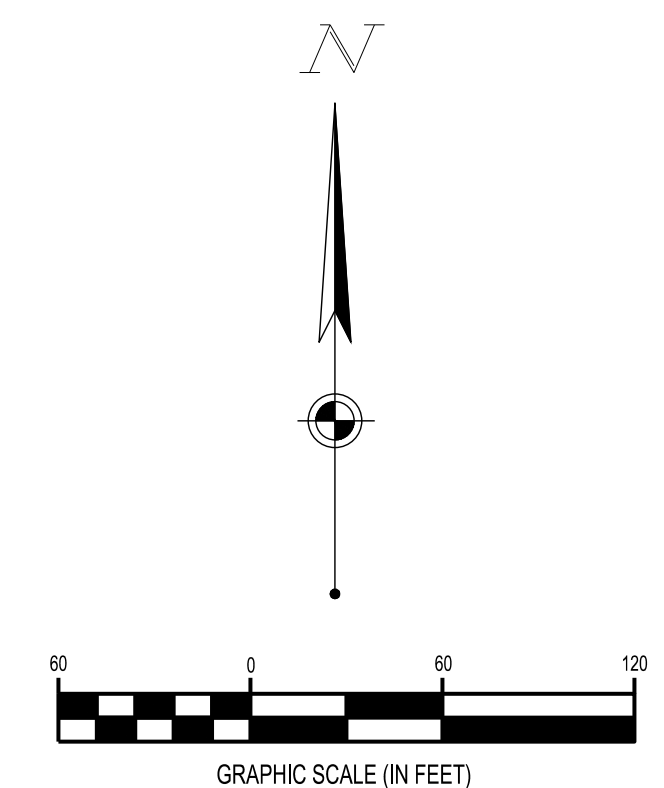
- ENSURE NOTICE OF INTENT (N.O.I.) IS FILED. KEEP A COPY OF THE PERMIT AND SWPPP ON SITE.
- INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SWPPP.
- INSPECTION OF EROSION CONTROL MEASURES AS OUTLINED IN NOTES. REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
- SITE DEMOLITION AND CLEARING.

NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION.

REFER TO SEDIMENT BASIN SKIMMER-OUTLET DETAIL ON SHEET C4.7



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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Revisions / Submissions		
ID	Description	Date

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 Project Number: 759267
 Scale: 1"=60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
SWPPP PHASE 1 - PREPARED PAD AREA

C4.4

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267_RV SWPPP.dwg - 5/10/2024 - Robert Herrmann



GENERAL NOTES

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- CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION, IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
- ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH THE OHIO DEPARTMENT OF NATURAL RESOURCES' RAINWATER AND LAND DEVELOPMENT MANUAL.
- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- REMOVE ALL ON SITE FEATURES AS SHOWN ON THE PLAN AND LEGALLY DISPOSE OF OFF SITE.
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LEGEND

- EXISTING**
- REFER TO ALTANSPS TOPOGRAPHIC SURVEY
 - MAJOR CONTOUR (1200)
 - MINOR CONTOUR (1199)
 - BUILDING
 - PAVEMENT
 - STM STORM SEWER
 - SF SILT FENCE
 - RIDGE LINE
 - GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
 - FLOW ARROW
 - TRUELOOK CAMERA
 - INLET PROTECTION (IP)
 - SOIL STABILIZATION (SS)
 - RIPRAP STORM SEWER OUTLET/SLOPE PROTECTION (RP)
 - TEMPORARY SEDIMENT BASIN (SB)
 - STORMWATER OUTFALL POINT (OF)
 - EROSION CONTROL BLANKET (EB)
 - STANDARD FLEXMAT (FM)
- PROPOSED**

REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
 REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN

SEQUENCE OF CONSTRUCTION - (PHASE TWO)

UNLESS NOTED OTHERWISE, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL MEASURES IN STRICT ACCORDANCE WITH THE SWPPP THROUGHOUT THE DURATION OF THE PROJECT.

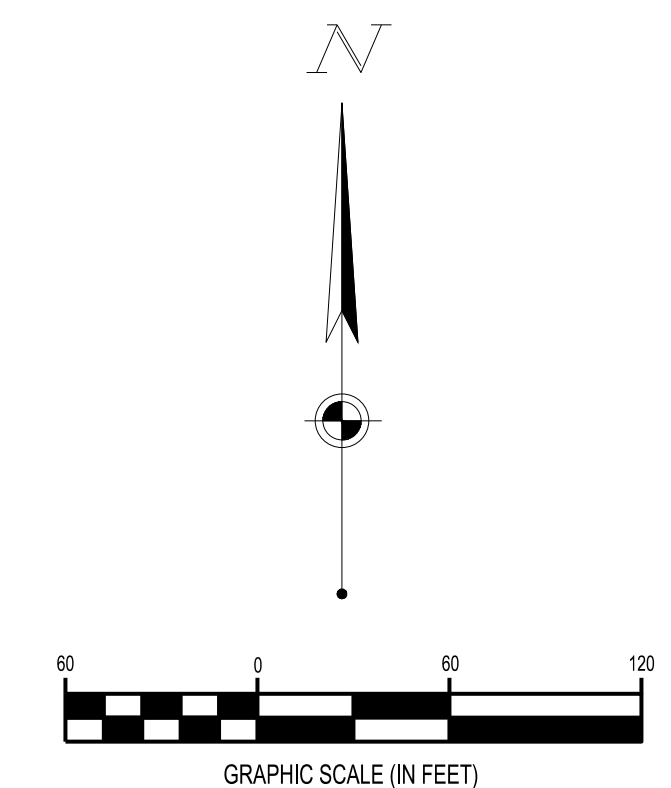
- INSPECTION OF EROSION CONTROL MEASURES INSTALLED UNDER PHASE ONE AS OUTLINED IN NOTES. REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
- INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SOIL EROSION PLAN FOR PHASE TWO.
- ROUGH GRADING. PROVIDE SOIL STABILIZATION OF DISTURBED AREAS WHICH ARE INACTIVE. REFER TO NPDES PERMIT REQUIREMENTS AND SWPPP DETAILS FOR ADDITIONAL INFORMATION.

NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION.

CODED NOTES

- TRUELOOK CAMERA.
- CONTRACTOR TO INSTALL EROSION CONTROL MEASURE FLEXMAT OR APPROVED EQUAL.

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2784 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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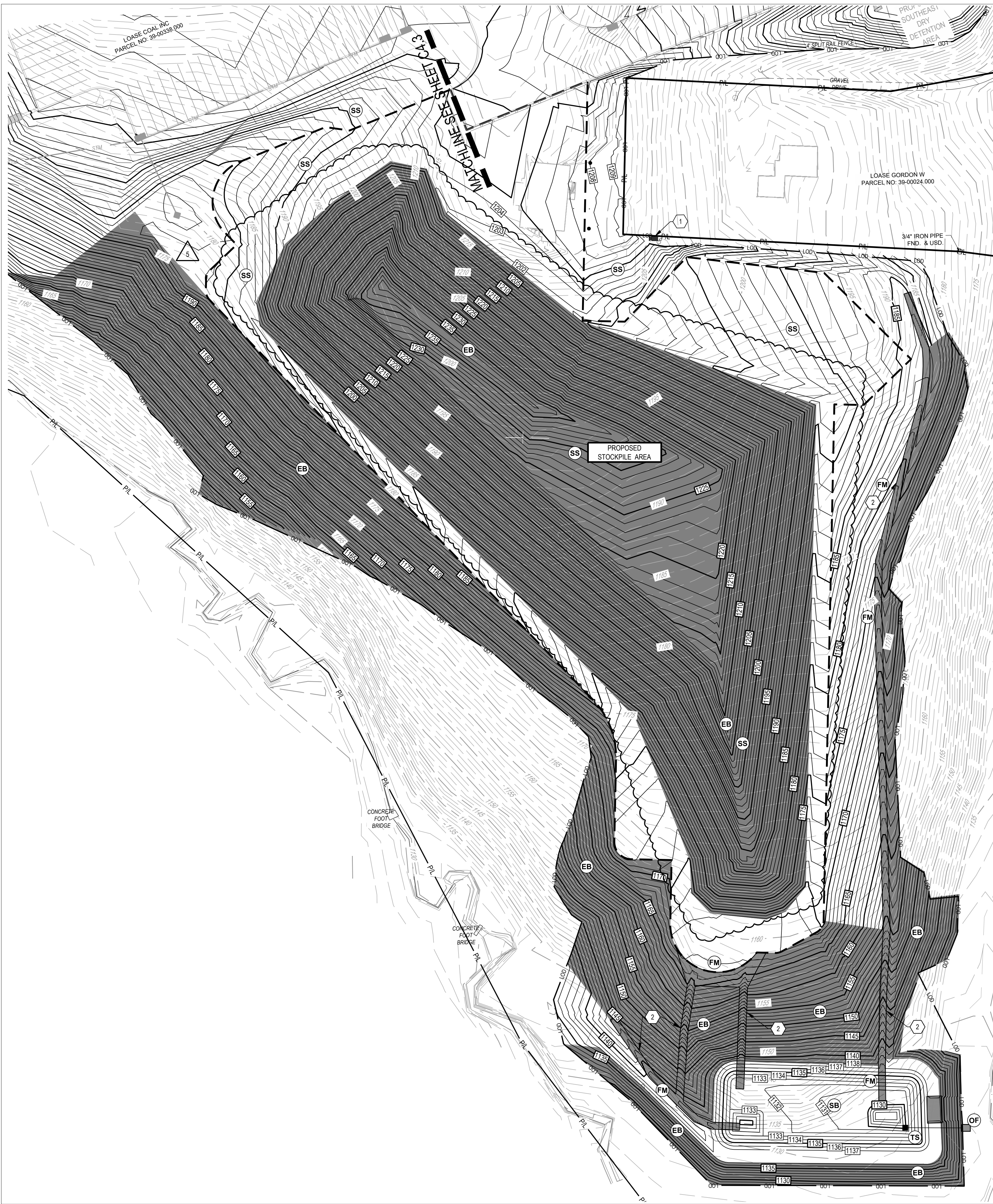
Revisions / Submissions		
ID	Description	Date
5	ADDENDUM 5	04/18/2024

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 Scale: 1"=60'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
**SWPPP PHASE 2 -
 PREPARED PAD AREA**

C4.5

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CODED NOTES

1. TRUELOOK CAMERA.
2. CONTRACTOR TO INSTALL EROSION CONTROL MEASURE FLEXAMAT OR APPROVED EQUAL.

LEGEND

- EXISTING**
- REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- 1200 MAJOR CONTOUR
 - 1199 MINOR CONTOUR
 - BUILDING
 - CONCRETE CURB AND GUTTER
 - PAVEMENT
 - STM STORM SEWER
 - RIDGE LINE
 - LOD GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
 - FLOW ARROW
 - FLOOD ROUTE
 - TRUELOOK CAMERA
 - SS SOIL STABILIZATION
 - RP RIPRAP STORM SEWER OUTLET/SLOPE PROTECTION
 - OF STORMWATER OUTFALL POINT
 - EB EROSION CONTROL BLANKET
 - FM STANDARD FLEXMAT
 - CE STABILIZED CONSTRUCTION ENTRANCE

REFER TO SHEET C4.0 FOR SWPPP GENERAL NOTES
 REFER TO SHEETS C4.7 TO C4.10 FOR SWPPP DETAILS
 REFER TO SHEETS C7 SERIES FOR GRADING PLAN

SEQUENCE OF CONSTRUCTION - (PHASE THREE)

UNLESS NOTED OTHERWISE, THE GENERAL CONTRACTOR IS RESPONSIBLE FOR MAINTAINING ALL EROSION AND SEDIMENT CONTROL MEASURES IN STRICT ACCORDANCE WITH THE SWPPP THROUGHOUT THE DURATION OF THE PROJECT.

1. INSPECTION OF EROSION CONTROL MEASURES INSTALLED UNDER PHASE ONE AND TWO AS OUTLINED IN NOTES. REPAIRS AND/OR REPLACEMENTS SHALL BE MADE AS NECESSARY.
2. INSTALL ALL TEMPORARY EROSION CONTROL MEASURES AS SHOWN ON THE SOIL EROSION PLAN FOR PHASE TWO.
3. REMOVE TEMPORARY RISER AND SKIMMER IN SEDIMENT BASIN, AND REMOVE ACCUMULATED SEDIMENT FROM BASIN, INLET INSERTS AND STORM STRUCTURE SUMPS FOR FINAL CONVERSION TO DRY BASIN.
4. PERMANENT BMP's.
5. FINAL STABILIZATION AND PERMANENT SEEDING. REFER TO THE NPDES REQUIREMENTS FOR ADDITIONAL INFORMATION. REFER TO LANDSCAPE PLAN FOR SOD LIMITS AND PLANTING AREAS.
6. REMOVE SOIL EROSION CONTROL MEASURES AFTER PERMANENT VEGETATION HAS BEEN ESTABLISHED. ENSURE NOTICE OF COMPLETION (N.O.C.) IS FILED.

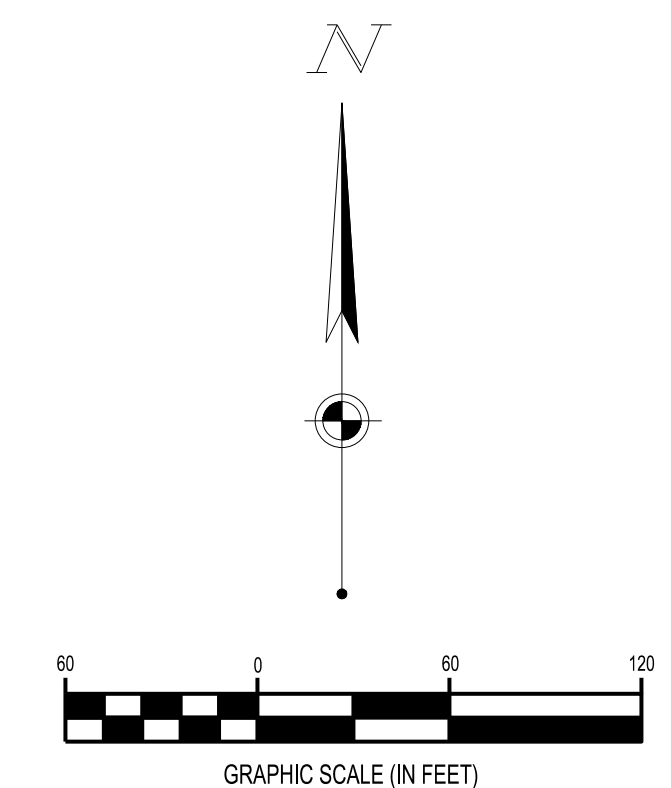
NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE THAT ALL SOIL EROSION CONTROL MEASURES ARE INSTALLED AND MAINTAINED DURING CONSTRUCTION. CONTRACTOR IS ALSO RESPONSIBLE FOR REMOVAL OF ALL TEMPORARY EROSION CONTROL MEASURES AFTER PERMANENT VEGETATION HAS BEEN ESTABLISHED.

GENERAL NOTES

1. ADDITIONAL EROSION AND SEDIMENT CONTROLS MAY BE REQUIRED AS IDENTIFIED WITH THE OHIO ENVIRONMENTAL PROTECTION AGENCY (OEPA) AND LOCAL JURISDICTION INSPECTOR.
2. CONTRACTOR SHALL REVIEW THE COMPLETE DRAWING SET AND NOTIFY THE DESIGN PROFESSIONAL IN WRITING PRIOR TO CONSTRUCTION, IF ANY DISCREPANCIES ARE FOUND WITHIN THE DRAWINGS OR WITH ACTUAL FIELD CONDITIONS.
3. ALL STORMWATER POLLUTION PREVENTION PLANS, NOTES AND DETAILS SHALL COMPLY WITH THE OHIO DEPARTMENT OF NATURAL RESOURCES' RAINWATER AND LAND DEVELOPMENT MANUAL.
4. CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
5. TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 6' BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS, OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2784 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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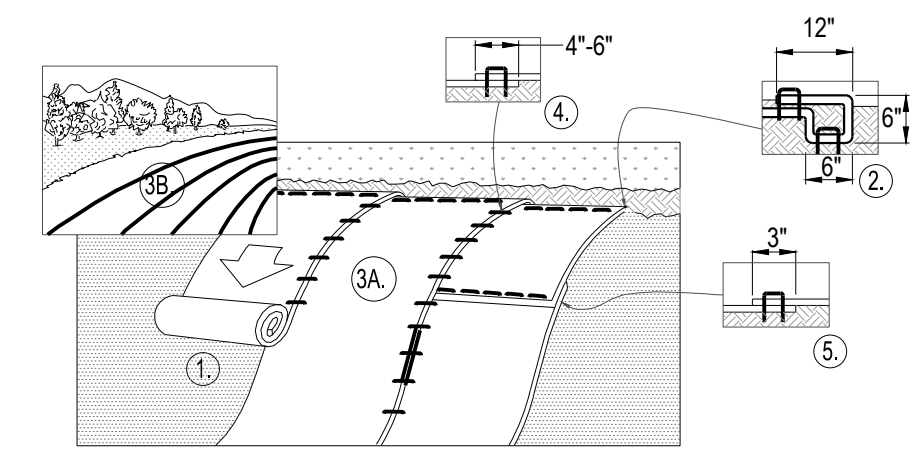
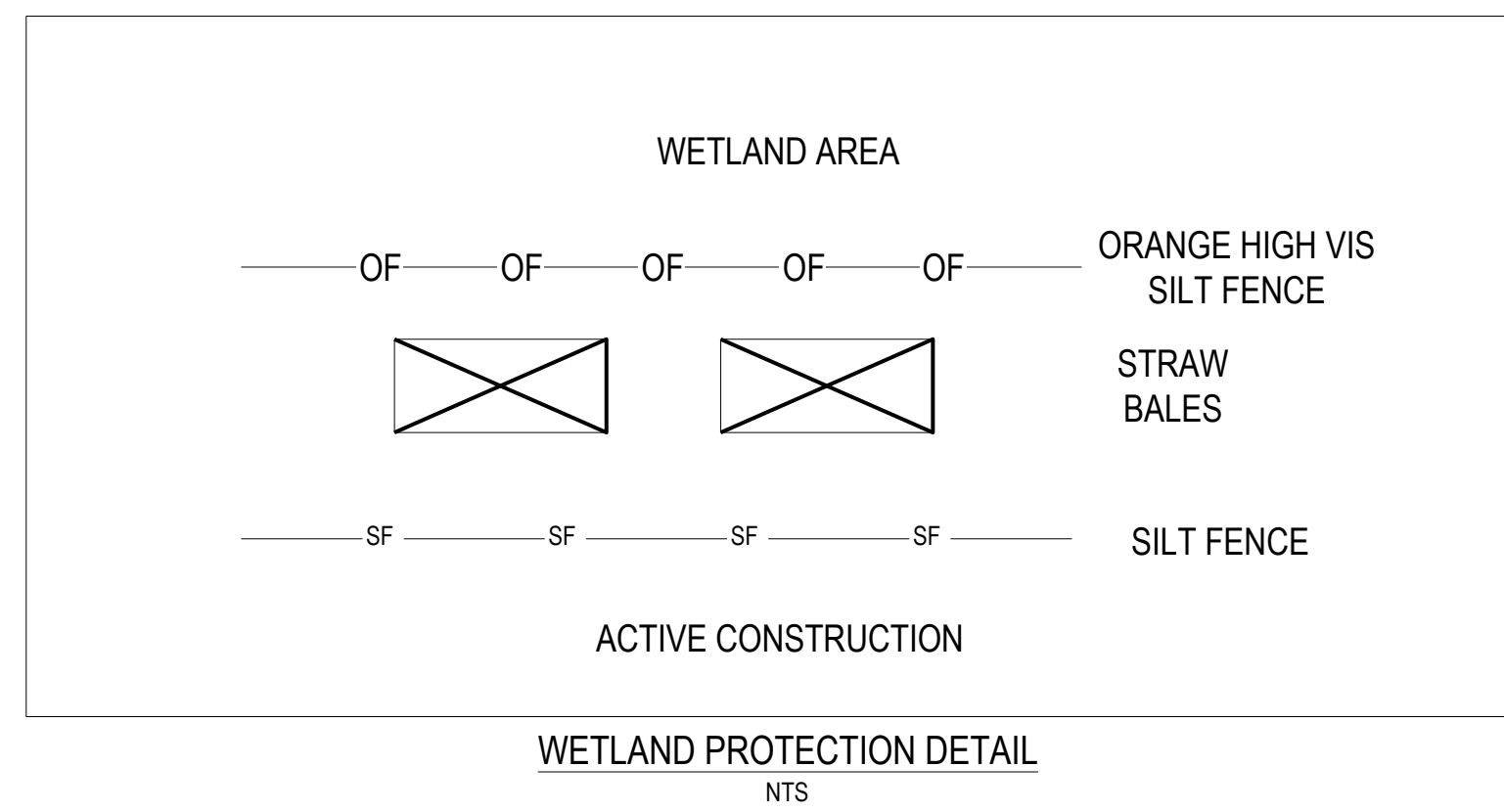
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ID	Description	Date
5	ADDENDUM 5	04/18/2024

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Drawing Title:
**SWPPP PHASE 3 -
 PREPARED PAD AREA**

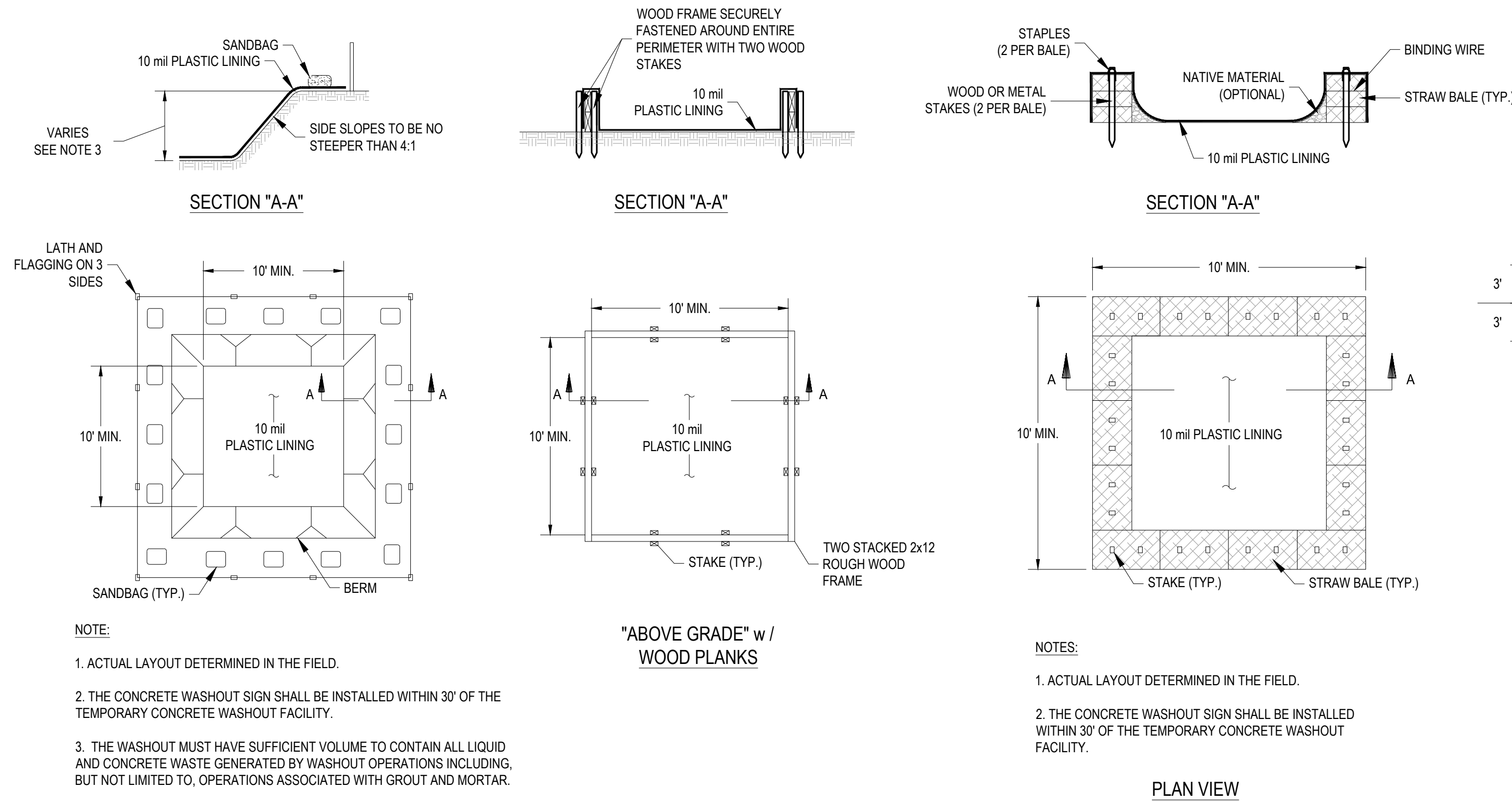
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- NOTES:**
1. PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING ANY NECESSARY APPLICATION OF LIME, FERTILIZER, AND SEED.
 2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN A 6" DEEP BY 6" WIDE TRENCH WITH APPROXIMATELY 12" OF BLANKET EXTENDED BEYOND THE UP-SLOPE PORTION OF THE TRENCH AS SHOWN IN DETAIL 2. ANCHOR THE BLANKET WITH A ROW OF STAPLES/STAKES APPROXIMATELY 12" APART IN THE BOTTOM OF THE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING. APPLY SEED TO COMPACTED SOIL AND FOLD REMAINING 12" PORTION OF BLANKET BACK OVER SEED AND COMPACTED SOIL. SECURE BLANKET OVER COMPACTED SOIL WITH A ROW OF STAPLES/STAKES SPACED APPROXIMATELY 12" APART ACROSS THE WIDTH OF THE BLANKET.
 3. ROLL THE BLANKETS (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE. BLANKETS WILL UNROLL WITH APPROPRIATE SIDE AGAINST THE SOIL SURFACE. ALL BLANKETS MUST BE SECURELY FASTENED TO SOIL SURFACE BY PLACING STAPLES/STAKES IN APPROPRIATE LOCATIONS AS PER MANUFACTURER'S RECOMMENDATION.
 4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH MINIMUM 6" OVERLAP. TO ENSURE PROPER SEAM ALIGNMENT, PLACE THE EDGE OF THE OVERLAPPING BLANKET (BLANKET BEING INSTALLED ON TOP) EVEN WITH THE SEAM STITCH ON THE PREVIOUSLY INSTALLED BLANKET.
 5. CONSECUTIVE BLANKETS SPLICED DOWN THE SLOPE MUST BE PLACED END OVER END (SHINGLE STYLE) WITH AN APPROXIMATE 3" OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" APART ACROSS ENTIRE BLANKET WIDTH.
 6. PLACE STAPLES/STAKES PER MANUFACTURER'S RECOMMENDATION FOR THE APPROPRIATE SLOPE BEING APPLIED.

EROSION CONTROL BLANKET
NTS

NOTE: CONTRACTOR MAY USE PRO MATRIX ENGINEERED FIBER MATRIX, OR APPROVED EQUAL, PER THE MANUFACTURER'S RECOMMENDED APPLICATION RATES, AS AN ALTERNATIVE TO EROSION CONTROL BLANKET IN AREAS OF 4:1 SLOPES OR GREATER. CONTRACTOR SHALL SUBMIT ALL NECESSARY SOIL SAMPLES AS REQUIRED BY THE MANUFACTURER IN ORDER TO DETERMINE THE CORRECT PRODUCT MIXTURE, AND A LOW MAINTENANCE SEED MIXTURE SHALL BE UTILIZED.



- NOTE:**
1. ACTUAL LAYOUT DETERMINED IN THE FIELD.
 2. THE CONCRETE WASHOUT SIGN SHALL BE INSTALLED WITHIN 30' OF THE TEMPORARY CONCRETE WASHOUT FACILITY.
 3. THE WASHOUT MUST HAVE SUFFICIENT VOLUME TO CONTAIN ALL LIQUID AND CONCRETE WASTE GENERATED BY WASHOUT OPERATIONS INCLUDING, BUT NOT LIMITED TO, OPERATIONS ASSOCIATED WITH GROUT AND MORTAR.

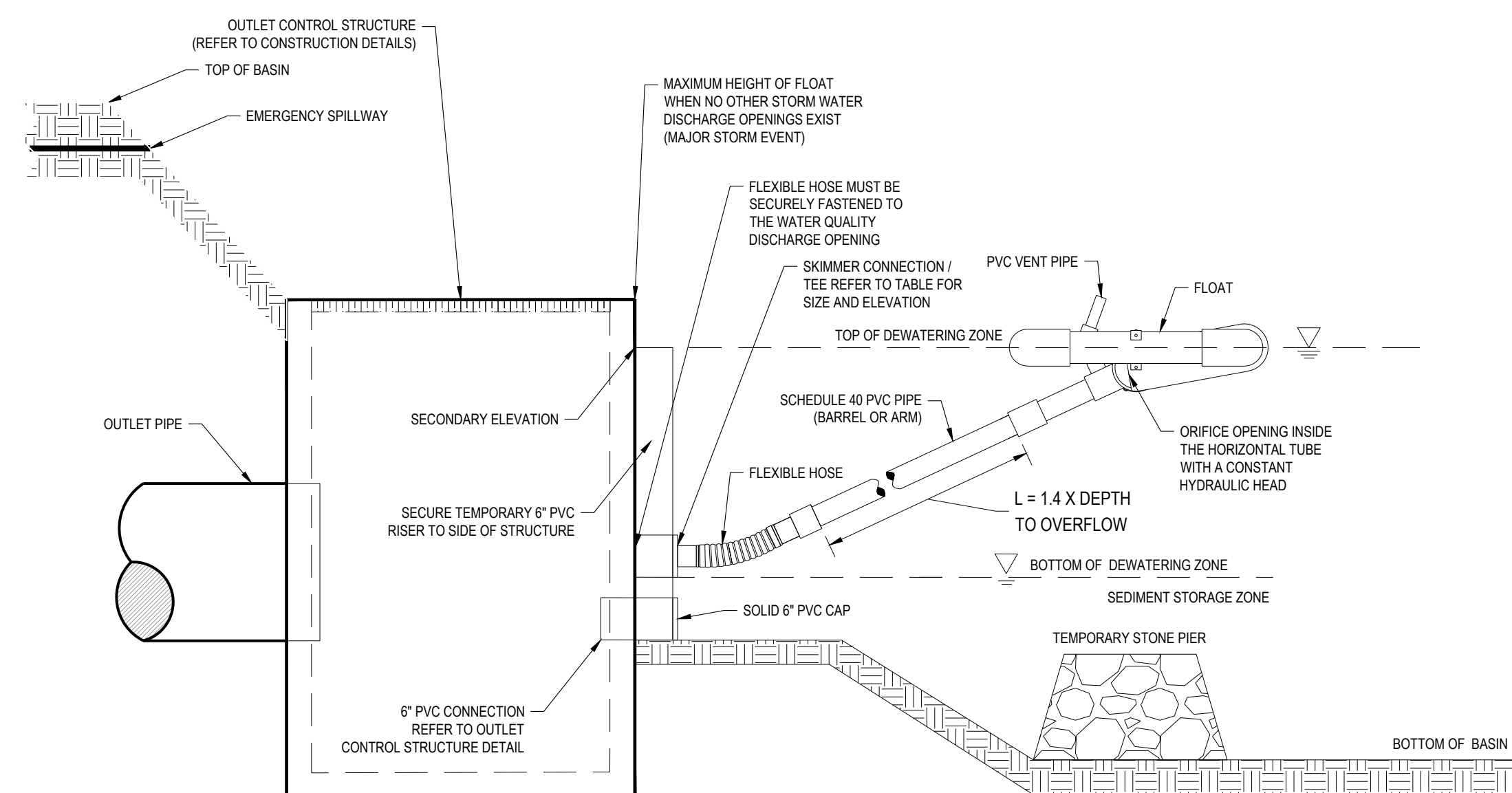
"BELOW GRADE"

"ABOVE GRADE" w / WOOD PLANKS

CONCRETE WASHOUT CW
NOT TO SCALE

- NOTES:**
1. ACTUAL LAYOUT DETERMINED IN THE FIELD.
 2. THE CONCRETE WASHOUT SIGN SHALL BE INSTALLED WITHIN 30' OF THE TEMPORARY CONCRETE WASHOUT FACILITY.

PLAN VIEW



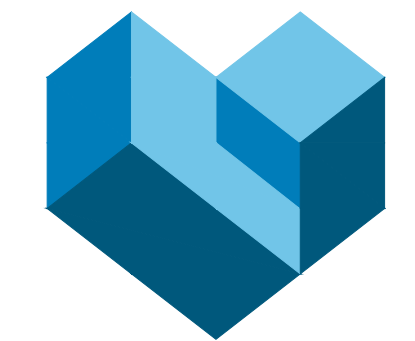
- NOTES:**
1. DIMENSIONS - REFER TO LIMITS OF EXCAVATION, SLOPES, PIPE INFORMATION, AND ADDITIONAL EROSION CONTROL MEASURES ON THE CONSTRUCTION DRAWINGS.
 2. ALL PIPE CONNECTIONS SHALL BE WATERTIGHT.
 3. INSPECT SYSTEM REGULARLY TO ENSURE IT IS FUNCTIONING IN A CORRECT MANNER.
 4. USE FAIRCLOTH SKIMMER OR APPROVED EQUAL.
 5. REMOVE SKIMMER UPON ACHIEVING FINAL STABILIZATION. INSTALL PERMANENT OUTLET PROTECTION / RIP RAP.
 6. REFER TO OUTLET STRUCTURE DETAIL FOR PERMANENT WATER QUALITY OR RUNOFF CONTROL MEASURES.
 7. AFTER SITE IS STABILIZED, REMOVE ACCUMULATED SEDIMENT FROM OUTLET STRUCTURE AND BASIN. REMOVE TEMPORARY RISER.
 8. INSTALL CONNECTION IN THE OUTLET STRUCTURE PER DETAIL FOR PERMANENT CONTROLS.

SEDIMENT BASIN SKIMMER-OUTLET STRUCTURE FOR PROPOSED TRAVEL STOP
NTS

	BASIN LOCATION			
	EAST	WEST	SOUTH EAST	SOUTH (PREPARED PAD)
DRAINAGE AREA (AC)	8.58	14.75	1.38	10.91
ELEVATION				
TOP OF BASIN	1195.00	1185.00	1175.00	1137.75
EMERGENCY SPILLWAY	1194.00	1184.00	1174.00	1136.75
SECONDARY ELEVATION	RIM=1192.25	WND=1192.08	RIM=1174.00	RIM=1134.00
SKIMMER OUTLET	1191.50	1181.25	1172.75	1132.30
BOTTOM OF BASIN	1190.00	1180.00	1171.00	1130.00
VOLUME (CF)				
SEDIMENT STORAGE REQUIRED	8,580	14,750	1,380	10,910
SEDIMENT STORAGE PROVIDED	9,700	16,334	2,028	11,149
DEWATERING - REQUIRED	15,444	26,550	2,484	19,638
DEWATERING - PROVIDED	16,193	27,561	6,565	22,861
SKIMMER SIZING				
DAYS TO DRAIN - REQUIRED	2	2	2	2
DAYS TO DRAIN - PROVIDED	2	2	2	2
FAIRCLOTH SKIMMER SIZE (IN)	4.00	5.00	2.00	4.00
FAIRCLOTH SKIMMER ORIFICE DIAMETER (IN)	3.10	4.00	1.50	3.70



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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12/18/2023

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Revisions / Submissions

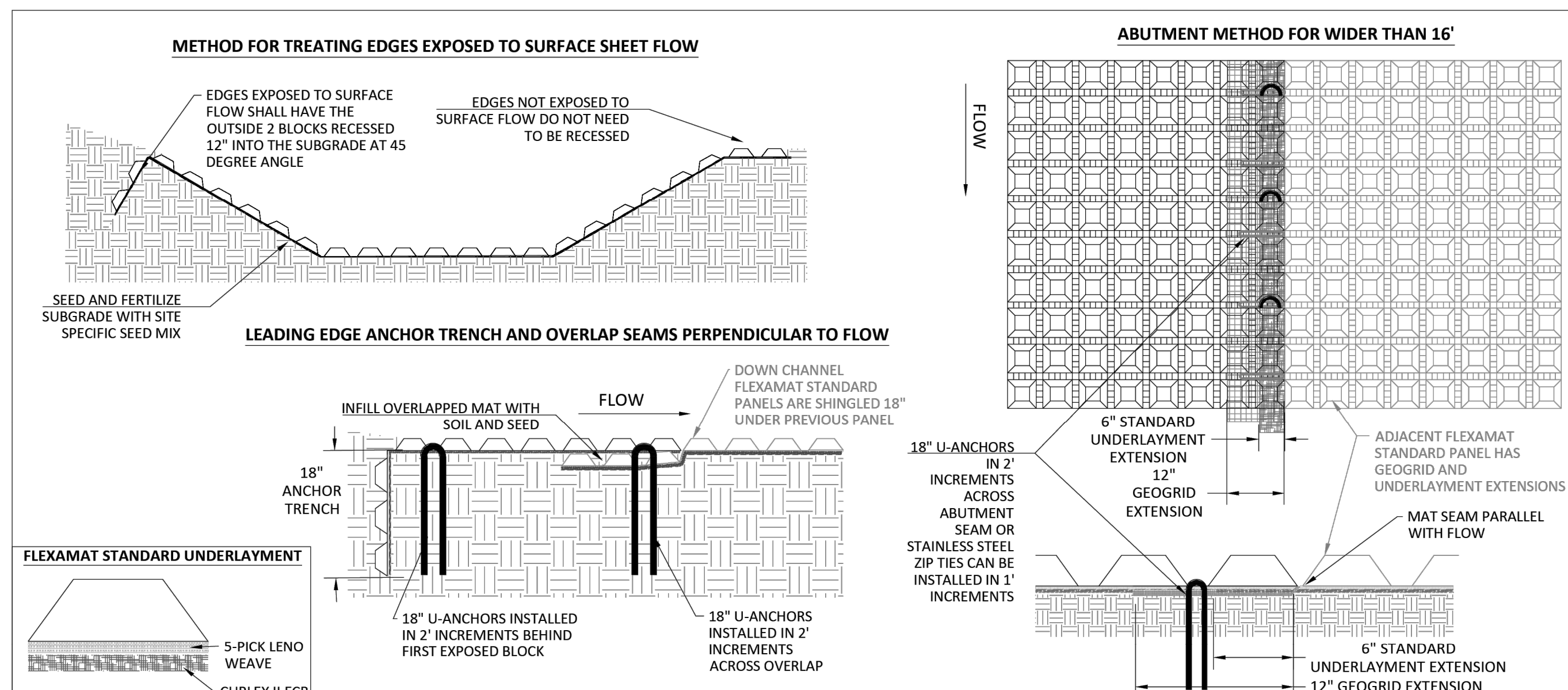
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SWPPP DETAILS

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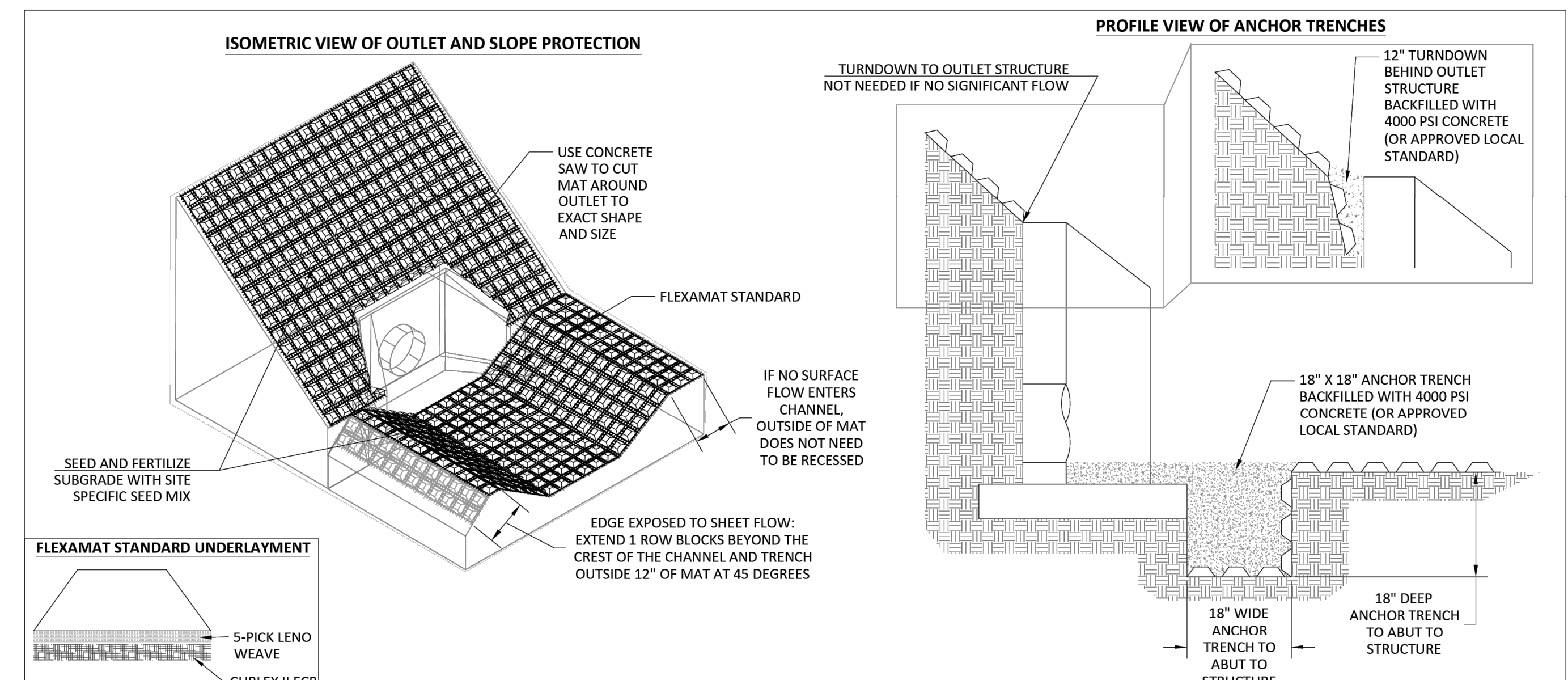
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FLEXAMAT STANDARD CHANNEL - LAYOUT PARALLEL TO FLOW

- CONSTRUCTION NOTES:**
- AN ENGINEER OR MANUFACTURERS REPRESENTATIVE SHALL BE ONSITE FOR THE START OF THE INSTALLATION.
 - GRADE CHANNEL SO THAT WATER WILL FLOW DOWN CENTER OF THE CHANNEL AND BE CONTAINED TO THE CHANNEL. ALL SUBGRADE SURFACES PREPARED FOR PLACEMENT OF MATS SHALL BE SMOOTH AND FREE OF ALL ROCKS, STICKS, ROOTS, OTHER PROTRUSIONS, OR DEBRIS OF ANY KIND.
 - PRIOR TO FLEXAMAT STANDARD INSTALLATION SEED AND FERTILIZE SUBGRADE WITH SITE SPECIFIC SEED MIX AND IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS.
 - INSTALL FLEXAMAT STANDARD ROLLS, USING THE WIDEST ROLLS POSSIBLE TO AVOID SEAMS.
 - FOR CHANNELS THAT ARE WIDER THAN 16', INSTALL 15.5' WIDE FLEXAMAT STANDARD ROLLS THAT INCLUDE 12" GEOGRID EXTENSIONS WITH A 6" STANDARD UNDERLAYMENT EXTENSIONS. THESE SEAMS ARE PARALLEL WITH FLOW, THE ADJACENT MAT INSTALLED OVER THE EXTENSIONS. ENSURE GEOGRID AND STANDARD UNDERLAYMENT EXTENSIONS ARE LAYING FLAT ON SUBGRADE PRIOR TO INSTALLING ADJACENT MAT.
 - SECURE THE ABUTMENT PARALLEL WITH FLOW BY INSTALLING 18" U-ANCHORS IN 2' INCREMENTS OR 20" STAINLESS STEEL ZIP TIES IN 1' INCREMENTS THROUGH THE EXTENSION OVERLAP. U-ANCHORS OR ZIP TIES TO BE INSTALLED PERPENDICULAR TO FLOW. ZIP TIES SHALL ENCOMPASS 3 CORDS OF GEOGRID FROM EACH MAT.
 - FOR ADDITIONAL SECTIONS OF MAT, SECURE SEAM PERPENDICULAR WITH FLOW BY OVERLAPPING THE DOWNSTREAM SECTION 18" WITH UPSTREAM SECTION OF MAT. PRIOR TO INSTALLING OVERLAP, FLIP UPSTREAM MAT BACK 24". EXCAVATE 2.25" OF SOIL 18" FROM END OF UPSTREAM MAT. DOWNSTREAM SECTION IS LAID IN THE SHALLOW TRENCH. RETURN AND TAMP SOIL OVER INITIAL EDGE AND SEED. FLIP END OF UPSTREAM MAT OVER THE SOIL COVERED AND SEED INITIAL LEADING EDGE OF DOWNSTREAM MAT.
 - SECURE OVERLAP PERPENDICULAR TO FLOW BY INSTALLING 18" U-ANCHORS IN 2' INCREMENTS OR 20" STAINLESS STEEL ZIP TIES IN 1' INCREMENTS THROUGH THE OVERLAP. ZIP TIES SHALL ENCOMPASS 3 CORDS OF GEOGRID FROM EACH MAT.
 - AT THE INITIAL LEADING EDGE OF THE FLEXAMAT STANDARD ARMORED CHANNEL, EMBED THE MAT 18" IN A VERTICAL ANCHOR TRENCH. FILL AND COMPACT ANCHOR TRENCH WITH SUITABLE FILL AT

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FLEXAMAT STANDARD - OUTLET ARMORING

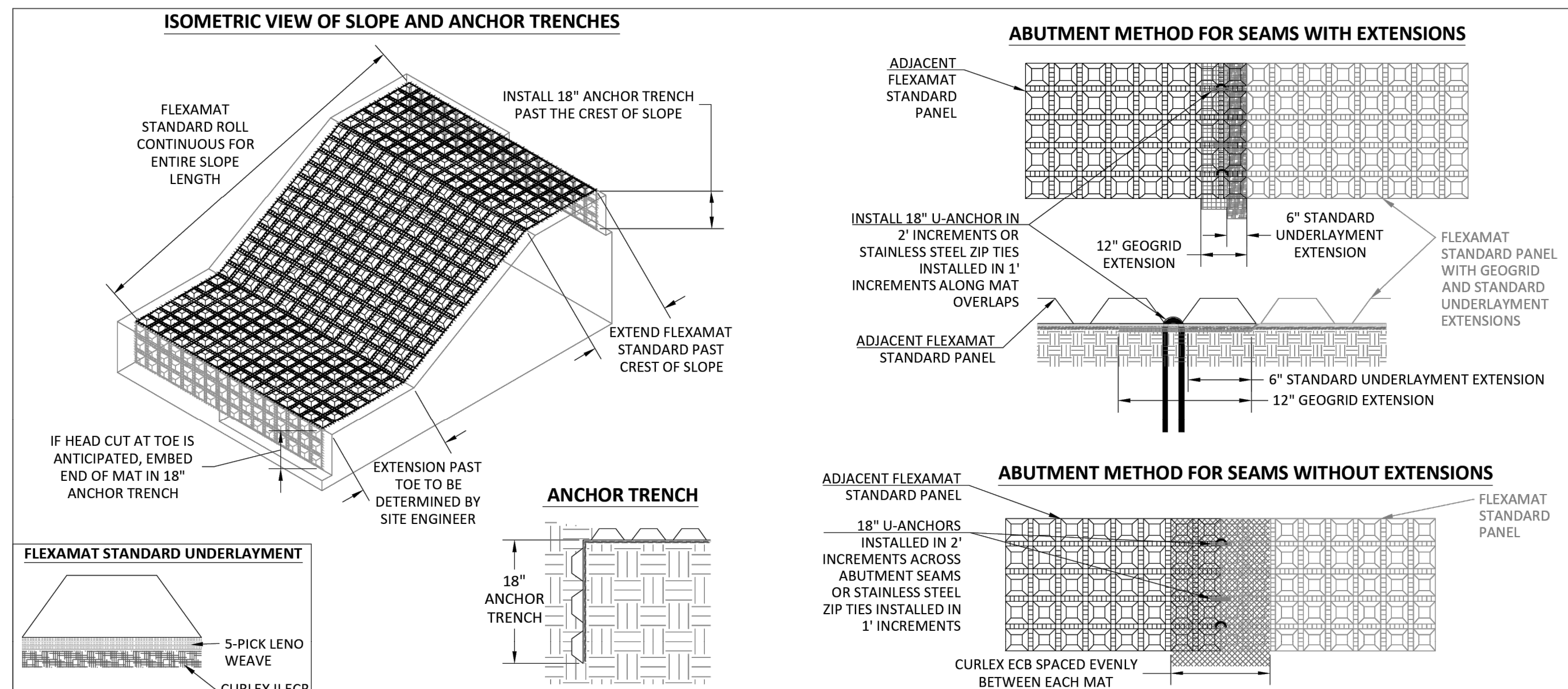
- CONSTRUCTION NOTES:**
- GRADE CHANNEL SO THAT WATER WILL FLOW DOWN CENTER OF THE CHANNEL AND BE CONTAINED TO THE CHANNEL. ALL SUBGRADE SURFACES PREPARED FOR PLACEMENT OF MATS SHALL BE SMOOTH AND FREE OF ALL ROCKS, STICKS, ROOTS, OTHER PROTRUSIONS, OR DEBRIS OF ANY KIND. THE PREPARED SURFACE SHALL PROVIDE A FIRM UNYIELDING FOUNDATION FOR THE MATS.
 - PRIOR TO FLEXAMAT STANDARD INSTALLATION, SEED AND FERTILIZE SUBGRADE WITH SITE SPECIFIC SEED MIX IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS.
 - INSTALL FLEXAMAT STANDARD ROLLS. AVAILABLE WIDTHS ARE 4', 5.5', 8', 10', 12', AND 16' AVAILABLE IN CUSTOM LENGTHS. FOR WIDTHS WIDER THAN 16', INSTALL 15.5' WIDE MAT WITH 12" GEOGRID EXTENSION AND 6" UNDERLAYMENT EXTENSION.
 - WHERE POSSIBLE AVOID LONGITUDINAL ABUTMENT SEAMS IN CHANNEL BOTTOM.
 - FOR OUTLET PROTECTION WIDER THAN 16' SEE CHANNEL PARALLEL TO FLOW INSTALLATION DETAIL.
 - FOR OUTLET PROTECTION THAT REQUIRES MORE THAN 1 MAT IN LENGTH TO COVER CHANNEL SEE CHANNEL PARALLEL TO FLOW INSTALLATION DETAIL.
 - AT THE BEGINNING OF CHANNEL, THE INITIAL LEADING EDGE OF FLEXAMAT EXPOSED TO CONCENTRATED FLOW SHALL BE EMBEDDED 18" VERTICALLY INTO ANCHOR TRENCH. THE TRENCH SHALL BE FILLED WITH 4,000 PSI CONCRETE.
 - AT THE END OF THE ARMORED CHANNEL, EMBED THE MAT 18" IN A TERMINATION TRENCH. FILL AND COMPACT TERMINATION TRENCH WITH A COHESIVE FILL.

GUIDANCE TABLE FOR STORMWATER OUTFALL PROTECTION

PIPE DIAMETER	FLEXAMAT WIDTH	FLEXAMAT LENGTH (*MIN)
12"	8 CFS	5'
18"	20 CFS	8'
24"	30 CFS	8'
36"	75 CFS	12'
48"	100 CFS	16'
60"	150 CFS	20'

*CONSULT MANUFACTURER FOR GUIDANCE IF DESIGN DISCHARGE VARIES SIGNIFICANTLY FROM VALUES LISTED IN TABLE.
 *LENGTH OF PROTECTION WILL VARY ON THE LENGTH OF THE SLOPE, OR IF IT IS DISCHARGING ONTO A FLAT AREA, OUTFALLS DISCHARGING ONTO SLOPES, IT IS RECOMMENDED TO EXTEND FLEXAMAT PLUS THE LENGTH OF THE SLOPE AND 3' PAST THE TOE.

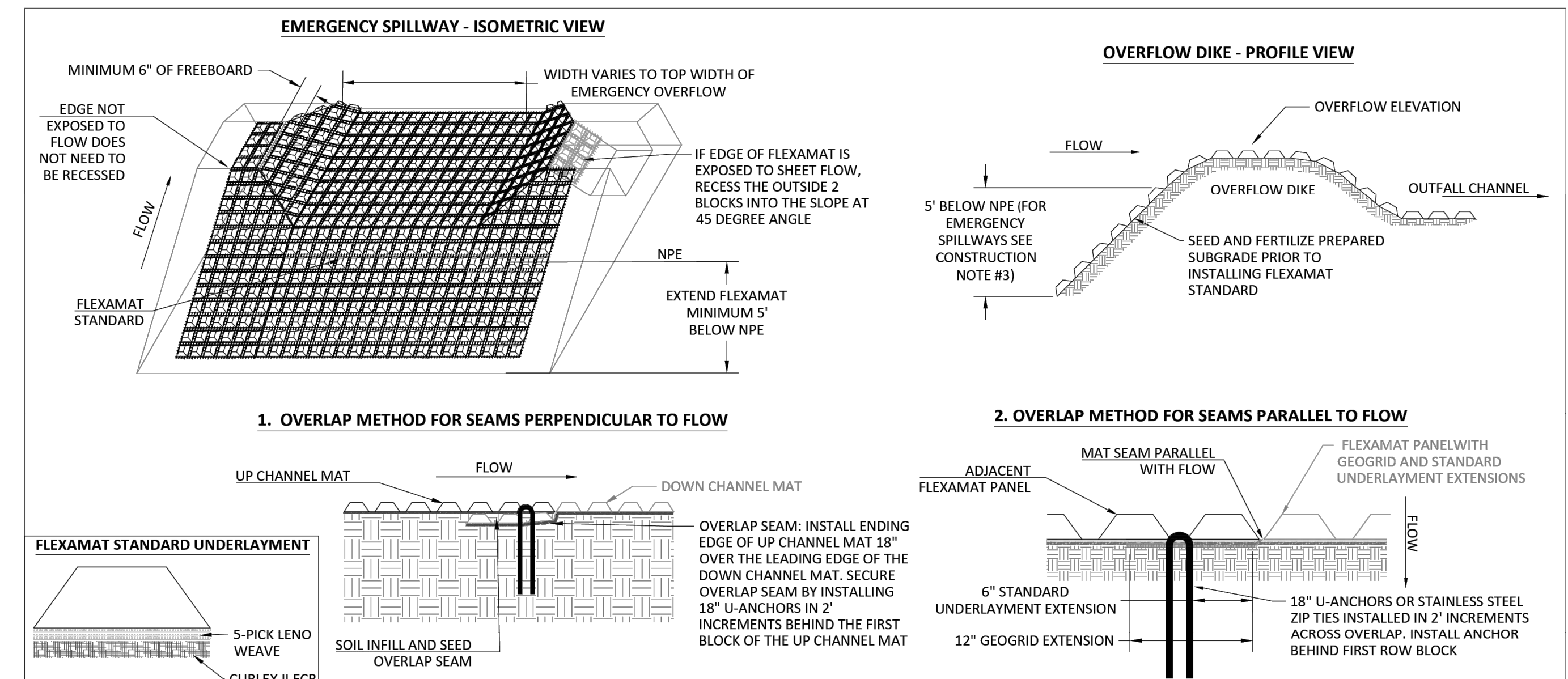
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FLEXAMAT STANDARD - SLOPE ARMORING

- CONSTRUCTION NOTES:**
- AN ENGINEER OR MANUFACTURERS REPRESENTATIVE SHALL BE ONSITE FOR THE START OF THE INSTALLATION.
 - ALL SUBGRADE SURFACES PREPARED FOR PLACEMENT OF MATS SHALL BE SMOOTH AND FREE OF ALL ROCKS, STICKS, ROOTS, OTHER PROTRUSIONS, OR DEBRIS OF ANY KIND.
 - PRIOR TO FLEXAMAT STANDARD INSTALLATION SEED AND FERTILIZE SUBGRADE WITH SITE SPECIFIC SEED MIX IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS.
 - INSTALL FLEXAMAT STANDARD ROLLS THAT ARE CONTINUOUS FOR ENTIRE SLOPE LENGTH. FOR SLOPES LONGER THAN 16', USE MATS WITH EXTENSIONS CUT TO THE LENGTH OF THE SLOPE. INSTALL MATS TO THAT THE MATTING EXTENDS PAST THE CREST OF SLOPE AND INTO AN 18" ANCHOR TRENCH.
 - FOR ARMORED SLOPE LENGTHS 16' OR LESS, INSTALL CURLEX ECB EQUALLY UNDER ADJACENT MATS. SECURE SEAM WITH #3 REBAR 18" U-ANCHORS IN 2' INCREMENTS THE LENGTH OF THE ABUTMENT.
 - ARMORED SLOPE LENGTHS LONGER THAN 16', INSTALL NEXT MAT OVER EXTENSIONS.
 - INSTALL SUBSEQUENT MATS OVER THE GEOGRID EXTENSION AND STANDARD UNDERLAYMENT EXTENSION OF THE PREVIOUSLY INSTALLED MAT. ENSURE THE GEOGRID AND STANDARD UNDERLAYMENT EXTENSIONS ARE LAYING FLAT ON THE SUBGRADE BEFORE INSTALLING ADJACENT MAT OVER THE EXTENSIONS.
 - INSTALL #3 REBAR 18" U-ANCHORS IN 2' INCREMENTS ACROSS THE GEOGRID AND STANDARD UNDERLAYMENT EXTENSION. INSTALL ANCHORS PERPENDICULAR TO THE SLOPE DIRECTLY BEHIND FIRST ROW OF BLOCKS ON THE ADJACENT MAT.
 - AT THE END OF THE ARMORED SLOPE, IF HEAD CUT IS ANTICIPATED, EMBED THE MAT 18" IN A TERMINATION TRENCH. FILL AND COMPACT TERMINATION TRENCH WITH SUITABLE FILL.

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FLEXAMAT STANDARD - OVERFLOW CHANNEL PARALLEL TO FLOW

- CONSTRUCTION NOTES:**
- AN ENGINEER OR MANUFACTURERS REPRESENTATIVE SHALL BE ONSITE FOR THE START OF THE INSTALLATION.
 - ALL SUBGRADE SURFACES PREPARED FOR PLACEMENT OF MATS SHALL BE SMOOTH AND FREE OF ALL ROCKS, STICKS, ROOTS, OTHER PROTRUSIONS, OR DEBRIS OF ANY KIND. THE PREPARED SURFACE SHALL PROVIDE A FIRM UNYIELDING SUBGRADE FOR THE MATS.
 - PRIOR TO THE FLEXAMAT STANDARD INSTALLATION SEED AND FERTILIZE SUBGRADE WITH SITE SPECIFIC SEED MIX IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS.
 - MAT SHALL EXTEND 5' BELOW NORMAL POND ELEVATION. (FOR EMERGENCY OVERFLOW INSTALLATIONS EXTEND THE MAT 3' DOWN THE INSIDE FACE OF THE OVERFLOW DIKE.)
 - INSTALL FLEXAMAT STANDARD ROLLS. MANUFACTURER RECOMMENDS INSTALLING THE WIDEST MAT POSSIBLE FOR SPILLWAY APPLICATIONS.
 - FOR WIDTHS WIDER THAN 16', INSTALL 15.5' WIDE MATS WITH GEOGRID AND STANDARD UNDERLAYMENT EXTENSIONS. INSTALL ADJACENT MAT OVER THE 12" GEOGRID AND 6" STANDARD UNDERLAYMENT EXTENSIONS OF THE ADJACENT MATS. ENSURE THE GEOGRID AND STANDARD UNDERLAYMENT EXTENSIONS ARE LAYING FLAT ON THE SUBGRADE BEFORE INSTALLING ADJACENT MAT OVER THE EXTENSIONS. INSTALL #3 REBAR 18" U-ANCHORS IN 2' INCREMENTS ACROSS THE GEOGRID AND STANDARD EXTENSION ABUTMENT. INSTALL ANCHORS PERPENDICULAR TO THE FLOW DIRECTLY BEHIND FIRST ROW OF BLOCKS ON THE ADJACENT MAT.
 - FOR ADDITIONAL SECTIONS OF MAT, OVERLAP THE DOWNSTREAM SECTION 18" WITH UPSTREAM SECTION OF MAT. PRIOR TO INSTALLING OVERLAP, FLIP UPSTREAM MAT BACK 24". EXCAVATE 2.25" OF SOIL 18" FROM THE END OF THE UPSTREAM MAT. DOWNSTREAM SECTION IS THEN LAID IN THE SHALLOW TRENCH. RETURN AND TAMP SOIL OVER INITIAL EDGE AND SEED AREA. FLIP END OF UPSTREAM MAT OVER THE SOIL COVERED INITIAL LEADING EDGE. SEED AND FERTILIZE SOIL INFILL PRIOR TO FLIPPING END OF UPSTREAM MAT OVER THE SOIL COVERED INITIAL LEADING EDGE OF DOWNSTREAM MAT. SECURE PERPENDICULAR OVERLAP SEAMS BY INSTALLING #3 REBAR - 18" U-ANCHORS PERPENDICULAR TO FLOW IN 2' INCREMENTS.
 - AT THE END OF THE ARMORED SPILLWAY, EMBED THE MAT 18" IN A TERMINATION TRENCH. FILL AND COMPACT TERMINATION TRENCH WITH SUITABLE FILL. (AS SPECIFIED BY EOR.)

MOTZ ENTERPRISES, INC.
 Flexamat
 (513)772-6689
 Info@Flexamat.com
 Flexamat.com



LOVES TRAVEL STOP
ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions

ID	Description	Date

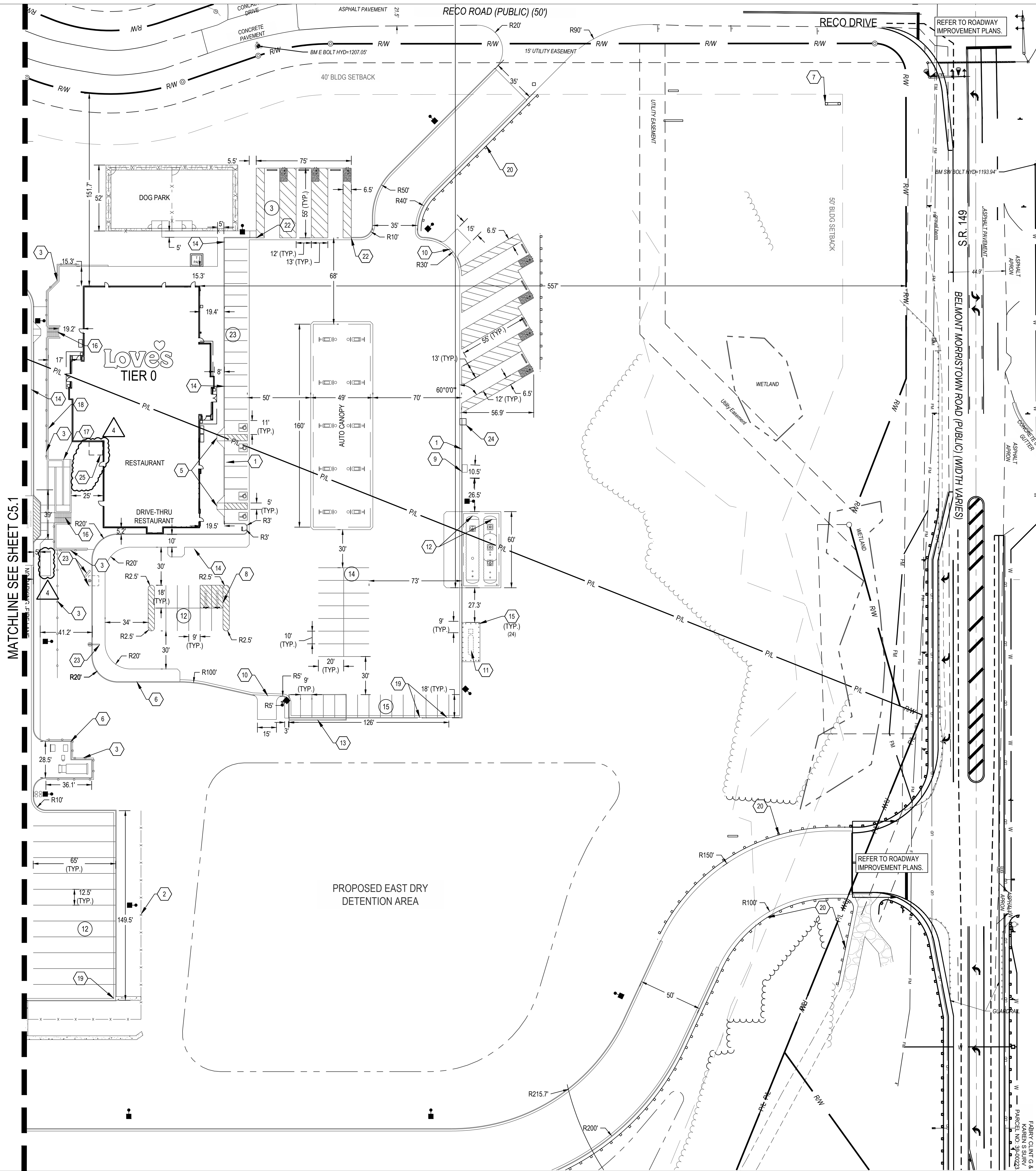
© 2023 CESO, INC.
 Project Number: 75927
 Scale: N/A
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
SWPPP DETAILS

Forty-eight (48) hours before digging is to commence, the contractors shall notify the following agencies: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

C4.10

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLANS\LOT-COMBINED\759267 SITE SITE.dwg - 4/12/2024 - Ryan Ash



LEGEND

EXISTING
REFER TO ALTANSPS TOPOGRAPHIC SURVEY

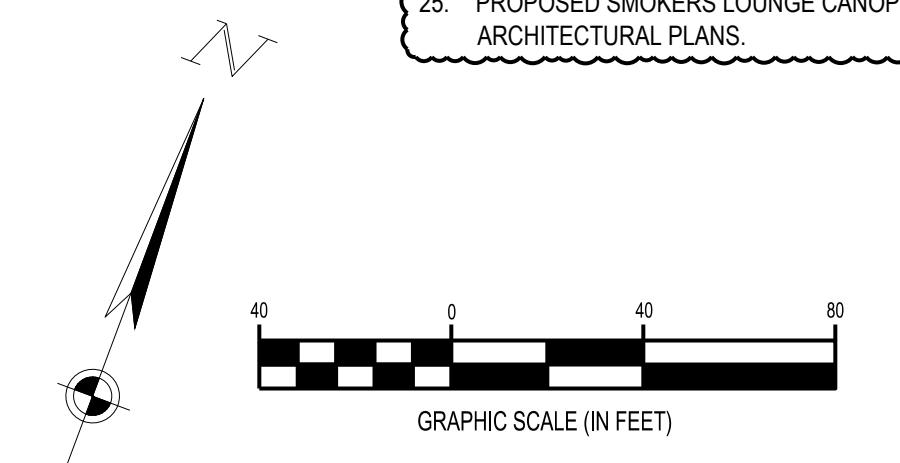
PROPOSED

- CONCRETE CURB AND GUTTER
- PAVEMENT
- BOLLARD
- 6 TREX FENCE
- 5 CHAIN-LINK FENCE
- 4 BLACK CHAIN-LINK-FENCE
- SETBACK
- XX' SBL
- (X) NUMBER OF PARKING STALLS
- GRAVEL
- PROPOSED TEMPORARY GRADING EASEMENT AREA

NOTE: REFER TO CONSTRUCTION DETAILS SHEETS

- GENERAL NOTES:**
- ALL WORK AND MATERIALS SHALL COMPLY WITH ALL ODOT STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL APPROVALS/PERMITTING AND INSPECTION AS NECESSARY PRIOR TO CONSTRUCTION.
 - ALL WET OR OTHERWISE UNSUITABLE SOILS MUST BE STABILIZED PRIOR TO PAVEMENT CONSTRUCTION. REFER TO GEOTECHNICAL REPORT.
 - ALL DIMENSIONS AND COORDINATES REFER TO EDGE OF PAVEMENT AND/OR FACE OF CURB WHERE APPLICABLE.
 - ALL RADII TO BE 5'-0" UNLESS OTHERWISE NOTED.
 - REFER TO CONSTRUCTION DETAILS SHEET/GEOTECHNICAL REPORT FOR PAVEMENT SECTION RECOMMENDATIONS.
 - REFER TO PAVEMENT PLAN FOR PAVEMENT DELINEATION.
 - REFER TO ARCHITECTURAL DRAWINGS FOR BUILDING LAYOUT AND DIMENSIONS AND PROPOSED SIGNS.
 - REFER TO ELECTRICAL DRAWINGS FOR LIGHT POLE LOCATIONS AND VERIFY BOLLARD PROTECTION WITH ARCHITECTURAL PLANS.

- CODED NOTES:**
- PROPOSED CURB AND GUTTER
 - PROPOSED 5' CHAIN-LINK FENCE. REFER TO ARCHITECTURAL PLANS
 - PROPOSED 6' TREX FENCE. REFER TO ARCHITECTURAL PLANS
 - STOP BAR
 - ADA RAMP
 - PROPOSED MONOLITHIC CURB
 - LOVE'S STREET SIGN. REFER TO ARCHITECTURAL PLANS.
 - RESERVED PARKING SPACE FOR DRIVE-THRU CUSTOMERS. REFER TO ARCHITECTURAL PLANS FOR SIGNAGE AND STRIPING.
 - AIR/ WATER ISLAND/ RV DUMP. REFER TO ARCHITECTURAL PLANS.
 - SNOW PLOT PUSH PAD. REFER TO DETAIL ON CONSTRUCTION DETAIL SHEET.
 - PROPANE STORAGE AREA. REFER TO ARCHITECTURAL PLANS.
 - FUEL STORAGE TANKS. REFER TO ARCHITECTURAL PLANS.
 - FUTURE EV CHARGING STALLS.
 - PROPOSED COLORED CONCRETE CURB. REFER TO PAVEMENT PLAN FOR LIMITS.
 - PROPOSED BOLLARD.
 - STAIRS.
 - SWITCH BACK RAMP. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
 - RETAINING WALL. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL INFORMATION.
 - PROPOSED CURB CUT.
 - PROPOSED GUARDRAIL.
 - PROPOSED JERSEY BARRIER.
 - TAPER CURB FROM 6" TO 0" OVER 1 FOOT.
 - PROPOSED DRIVE THRU EQUIPMENT. REFER TO ARCHITECTURAL PLANS FOR DETAILS FOR ADDITIONAL INFORMATION.
 - PROPOSED RV SIGNAGE. REFER TO ARCHITECTURAL PLANS.
 - PROPOSED SMOKERS LOUNGE CANOPY. REFER TO ARCHITECTURAL PLANS.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

ID	Description	Date
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

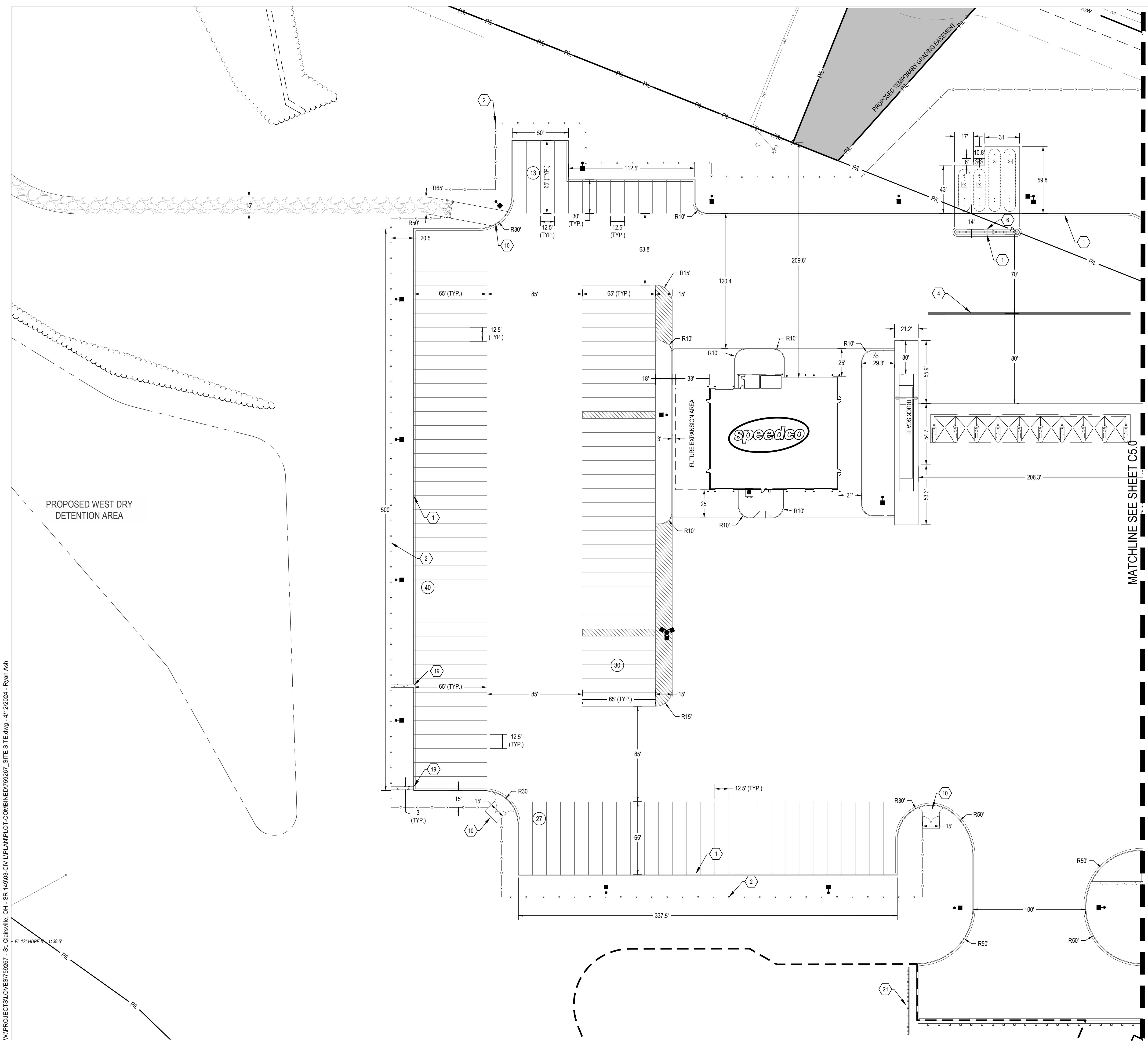
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Project Number: 759267
 Scale: 1" = 40'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
SITE PLAN - EAST

C5.0

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LEGEND

EXISTING
REFER TO ALTANSPS TOPOGRAPHIC SURVEY

PROPOSED

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- PAVEMENT
- BOLLARD
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- NUMBER OF PARKING STALLS
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 - PROPOSED SMOKERS LOUNGE CANOPY. REFER TO ARCHITECTURAL PLANS.

40 0 40 80
GRAPHIC SCALE (IN FEET)

OHIO811.org
Before You Dig

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

CESO
WWW.CESONC.COM

175 Monroe West Ave., Suite 400
Akron, OH 44321
Phone: 330.665.0660 Fax: 888.208.4826

LOVE'S TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

ID	Description	Date
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

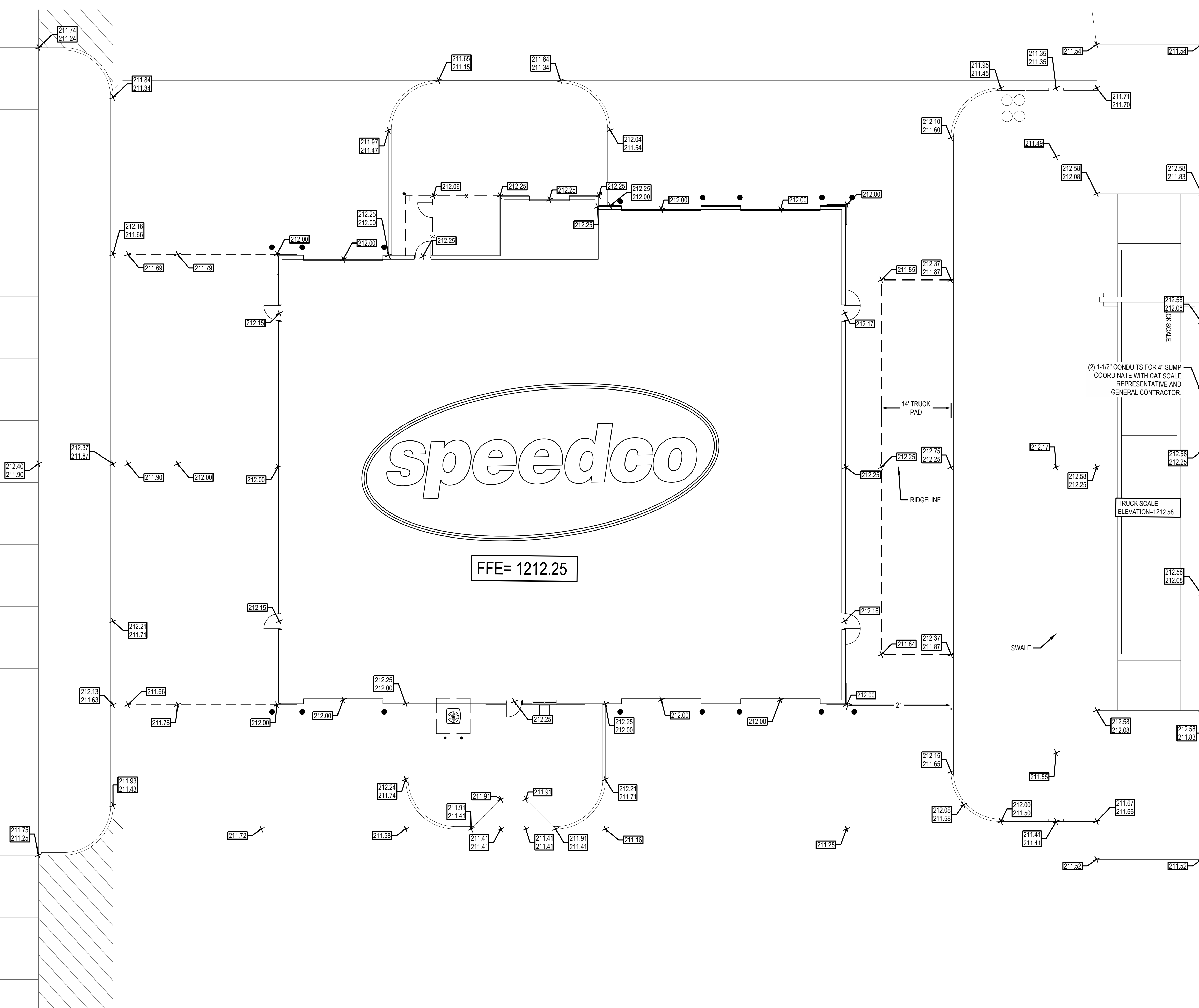
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Project Number: 759267
Scale: 1" = 40'
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
SITE PLAN - WEST

C5.1

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLANS\LOT-COMBINED\759267 SITE TIRE SHOP GRADING DETAIL.dwg - 12/18/2023 - Francesca Rappaselli



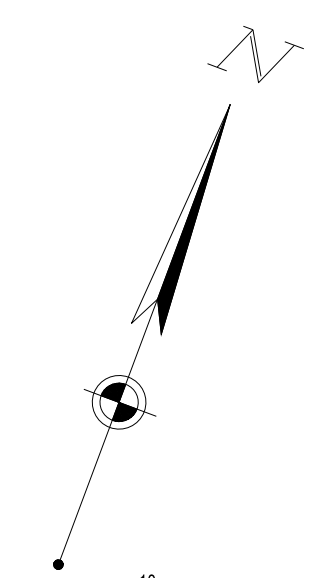
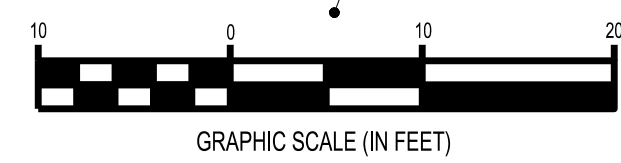
TIRE SHOP DETAIL
SCALE: 1" = 10'

NOTES

1. ALL SPOT GRADES REFER TO TOP OF PAVEMENT UNLESS OTHERWISE NOTED.
2. SEE GRADING PLAN FOR ADDITIONAL GRADES ON SITE.
3. ALL DISCREPANCIES BETWEEN SPOT ELEVATIONS ON THIS SHEET AND THE GRADING PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE AND THE DESIGN ENGINEER.
4. ELEVATIONS SHOWN AT TIRE SHOP DOORS SHOULD BE VERIFIED WITH ARCHITECTURAL PLANS. ANY DISCREPANCIES BETWEEN THIS SHEET AND THE ARCHITECTURAL PLANS SHOULD BE BROUGHT TO THE ATTENTION OF THE OWNER'S REPRESENTATIVE AND THE DESIGN ENGINEER.
5. THE TIRE SHOP FINISHED FLOOR ELEVATION (1212.25) IS EQUAL TO THE ARCHITECTURAL FINISHED FLOOR ELEVATION 100.00 NOTED ON THE FOUNDATION PLANS.
5. ADD 1000.00 TO ALL SPOT ELEVATIONS SHOWN ON PLAN TO OBTAIN U.S.G.S. ELEVATIONS.

(2) 1-1/2" CONDUITS FOR 4" SUMP COORDINATE WITH CAT SCALE REPRESENTATIVE AND GENERAL CONTRACTOR.

TRUCK SCALE ELEVATION=1212.58



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CESO
WWW.CESOINC.COM

175 Monrovia West Ave., Suite 400
Alvord, OH 44321
Phone: 330.665.0960 Fax: 888.208.4826



12/18/2023

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

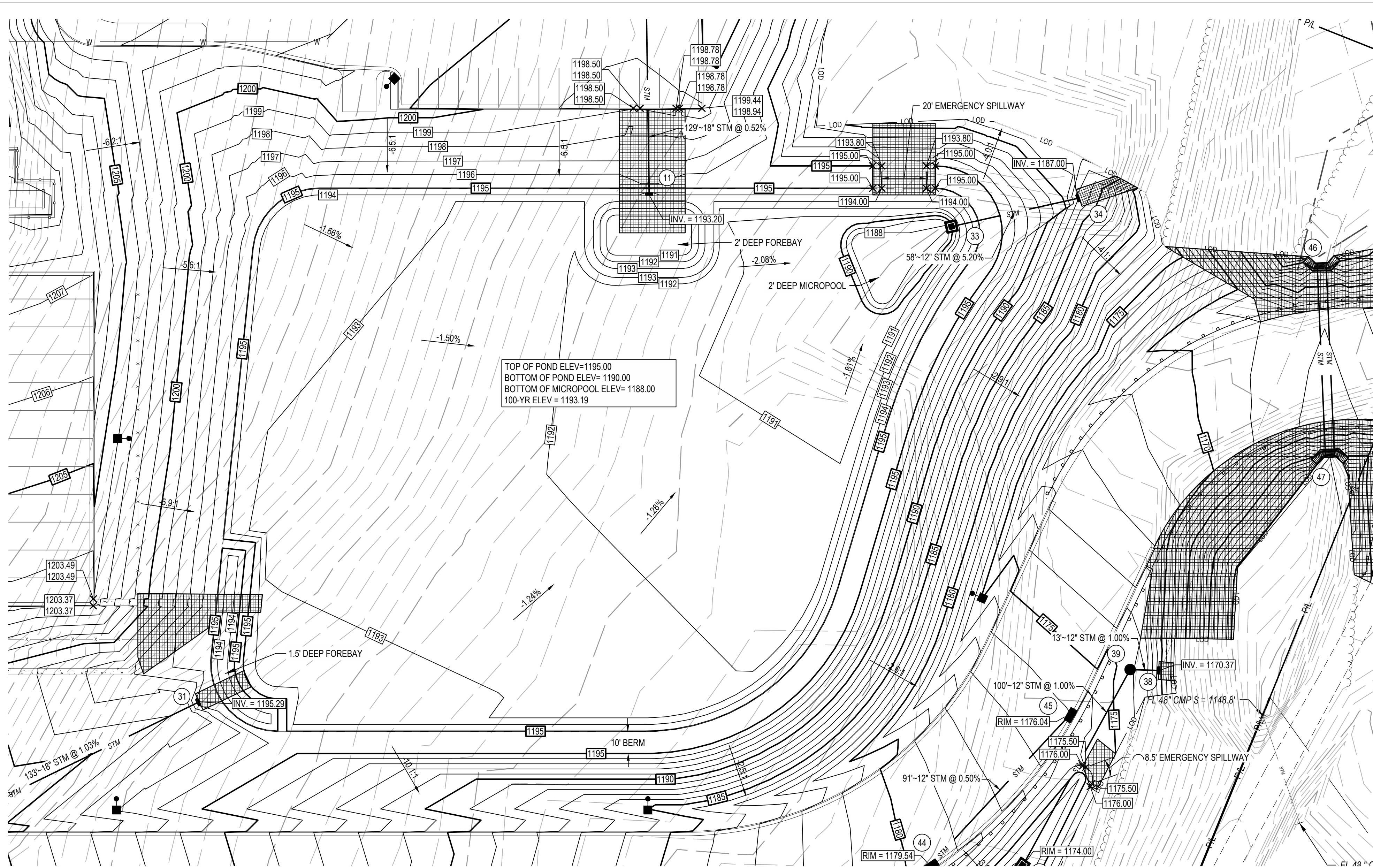
Revisions / Submissions

ID	Description	Date

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 Project Number: 759267
 Scale: 1" = 10'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
TIRE SHOP GRADING DETAIL

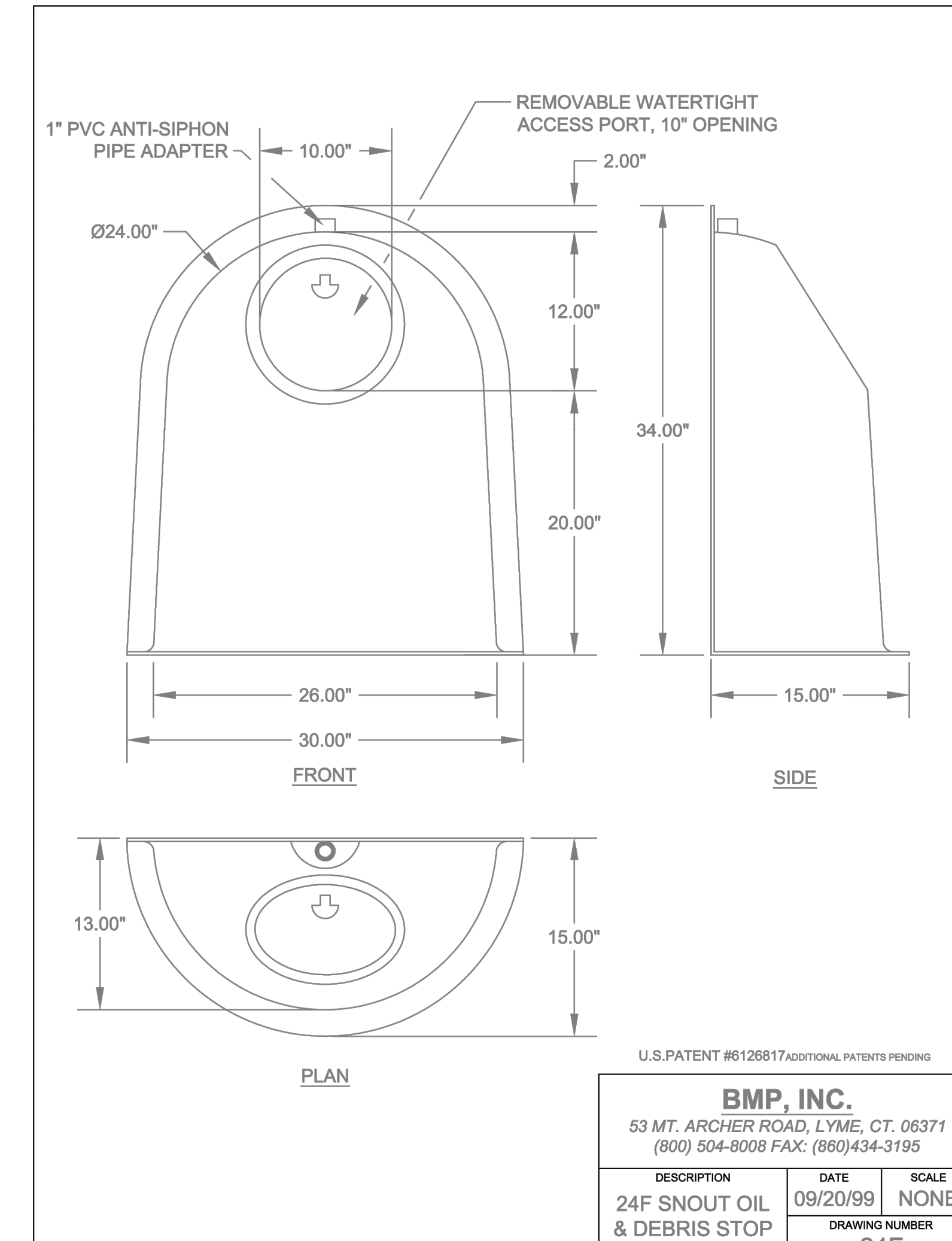
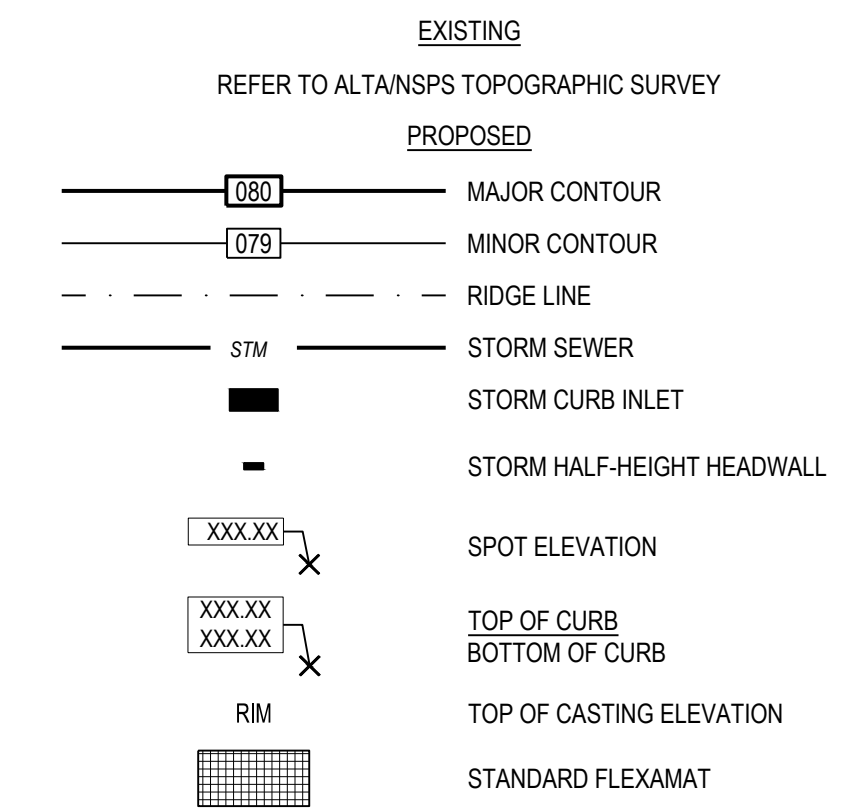
C7.8



GENERAL NOTES

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- STORM SEWER CATCH BASINS, CURB INLETS, MANHOLES AND ENDWALLS SHALL CONFORM TO ODOT STANDARD CONSTRUCTION DRAWINGS.
- REFER TO SWPPP PLANS FOR RIPRAP / FLEXAMAT LOCATIONS AND DIMENSIONS AND ALL OTHER PERMANENT EROSION CONTROL MEASURES.

LEGEND



East Dry Detention Pond Volume

Elevation	Contour Area (SF)	Incremental Volume (CF)	Total Volume (CF)
1188	774	0	0
1189	1,046	907	907
1190	1,352	1,196	2,103
1191	7,520	4,020	6,123
1192	23,109	14,604	20,727
1193	51,812	36,508	57,235
1194	65,232	58,393	115,628
1195	68,466	66,842	182,470

East Drainage Area Stormwater Release Rate Summary

Storm Frequency	Pre-Developed Peak Flow (CFS)	Dry Detention Basin Peak Elevation	Dry Detention Basin Peak Flow (CFS)	Total Post-Developed Peak Flow (CFS)
2	1.76	1192.52	0.27	1.47
5	5.37	1192.61	1.05	2.62
10	8.99	1192.70	2.29	3.64
25	14.98	1192.86	4.70	8.32
50	20.28	1193.02	5.60	11.26
100	26.31	1193.19	6.21	13.57

East Basin Spillway Calculations

(Q100) Estimated 100-yr Flow = **50.9** cfs

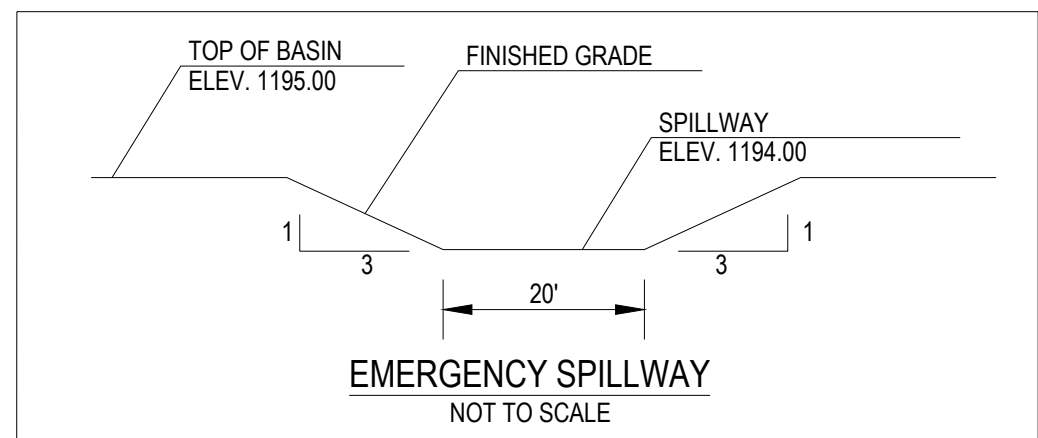
(Qs) Spillway Capacity = $C_w \cdot L \cdot (h^{3/2})$

Spillway Coefficient, $C_w = 2.90$

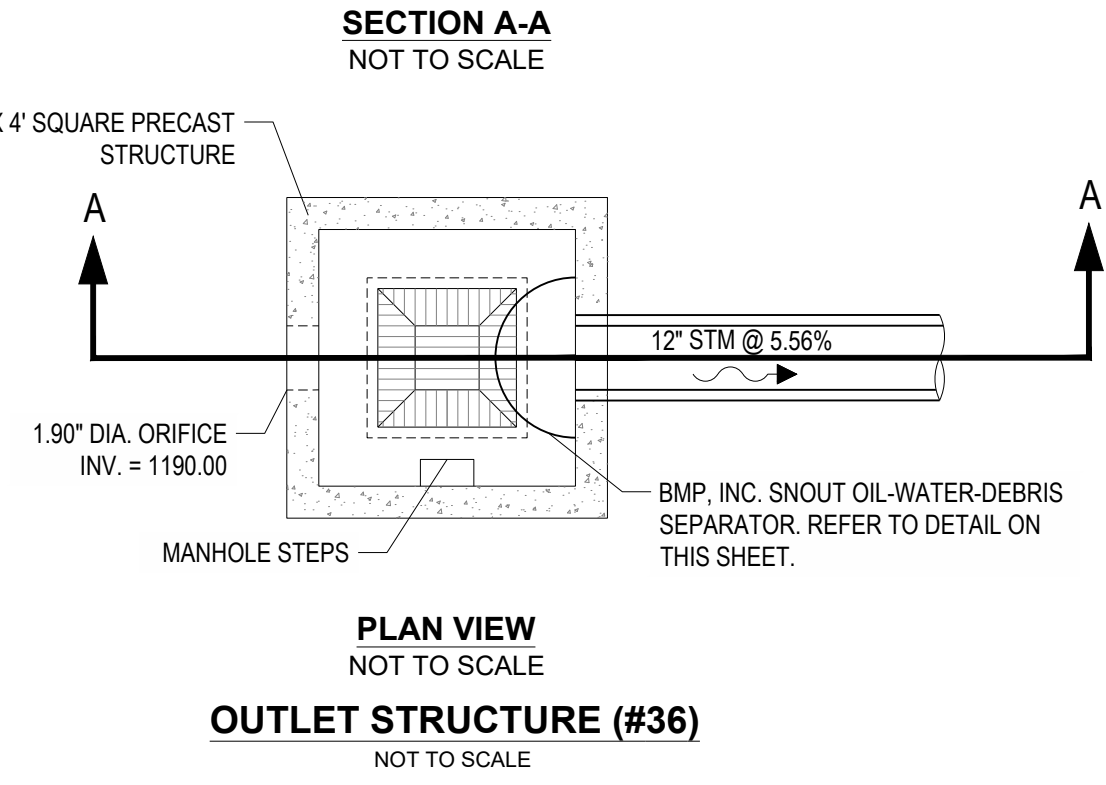
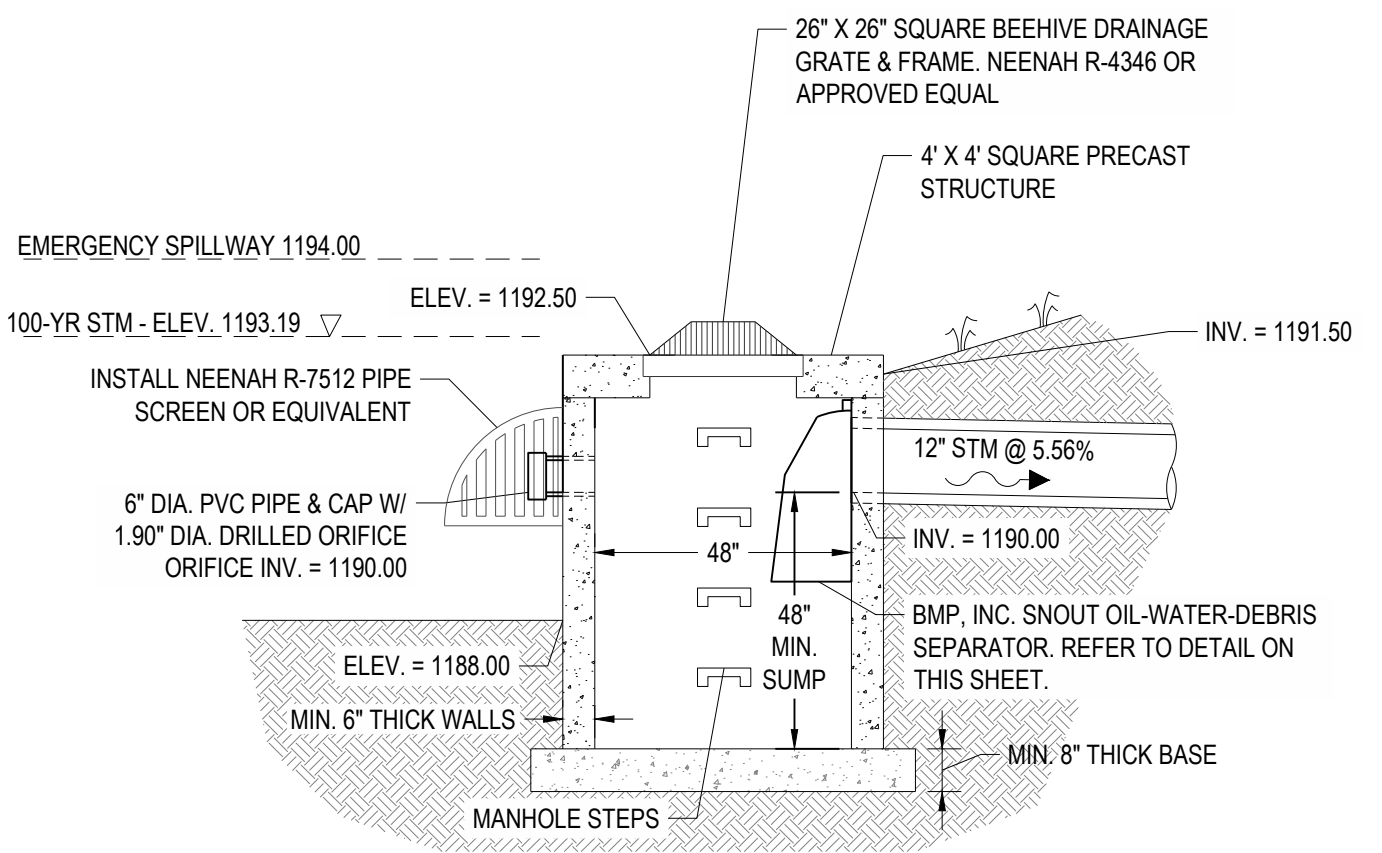
Bottom Width, $L = 20.00$ ft

Height, $h = 1.00$ ft

$Q_s = 58.00$ cfs



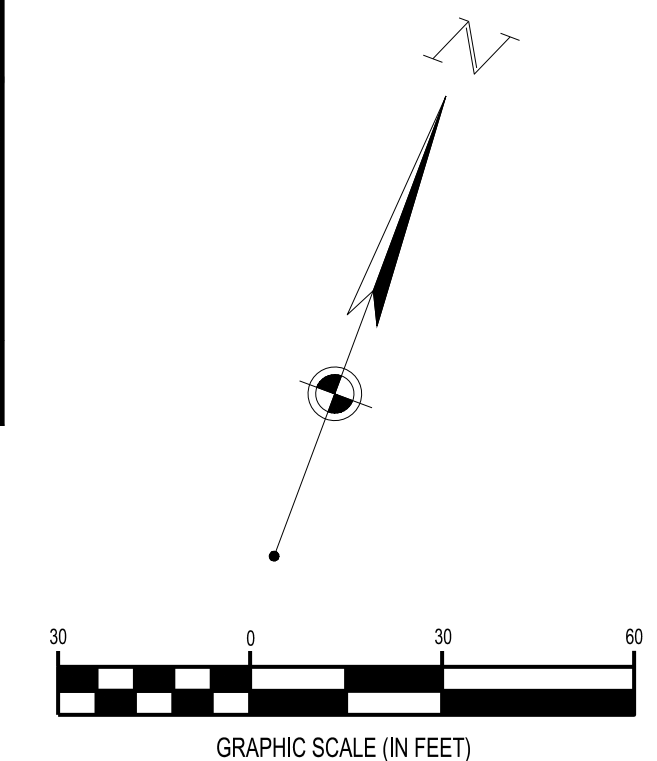
STORMWATER CALCULATIONS BASED ON ODOT REGULATIONS. THE POST-DEVELOPED PEAK RUNOFF RATE FOR THE 2-YR. THROUGH 100-YR RETURN PERIOD STORM MUST NOT EXCEED THE PRE-DEVELOPED PEAK RUNOFF RATE FOR THE EQUIVALENT STORM RETURN PERIOD.



BMP WATER QUALITY STRUCTURE NOTICE

THE OUTLET STRUCTURE IS AN INTEGRAL PART OF THE PRIVATE STORM SEWER SYSTEM DEPICTED IN THESE DRAWINGS. RESPONSIBILITY AND ASSURANCE OF PERIODIC MAINTENANCE AND THE CONTINUOUS FUNCTIONALITY OF THESE STORMWATER QUALITY DEVICES IS PERPETUAL, BEGINNING WITH THE OWNER AT THE TIME OF INSTALLATION AND CONTINUING TO ALL FUTURE OWNERS OF SAID PRIVATE STORM SEWER SYSTEM.

CONTRACTOR TO ENSURE THAT THE DETENTION SYSTEM (INCLUDING ALL DETENTION STRUCTURES AND WATER QUALITY STRUCTURES) IS CLEAN AND FUNCTIONS PROPERLY BEFORE SYSTEM IS TURNED OVER TO THE OWNER.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE

CESO logo with website URL www.ceso-inc.com. Below it is the Loves logo, which features a red heart and the word "Loves" in a stylized font.

STATE OF OHIO seal for ZACHARY D. FRESHNER, E-73224, dated 12/18/2023.

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions table:

ID	Description	Date

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Project Number: 759267

Scale: 1" = 30'

Drawn By: FAR

Checked By: JTK

Date: 12/19/2023

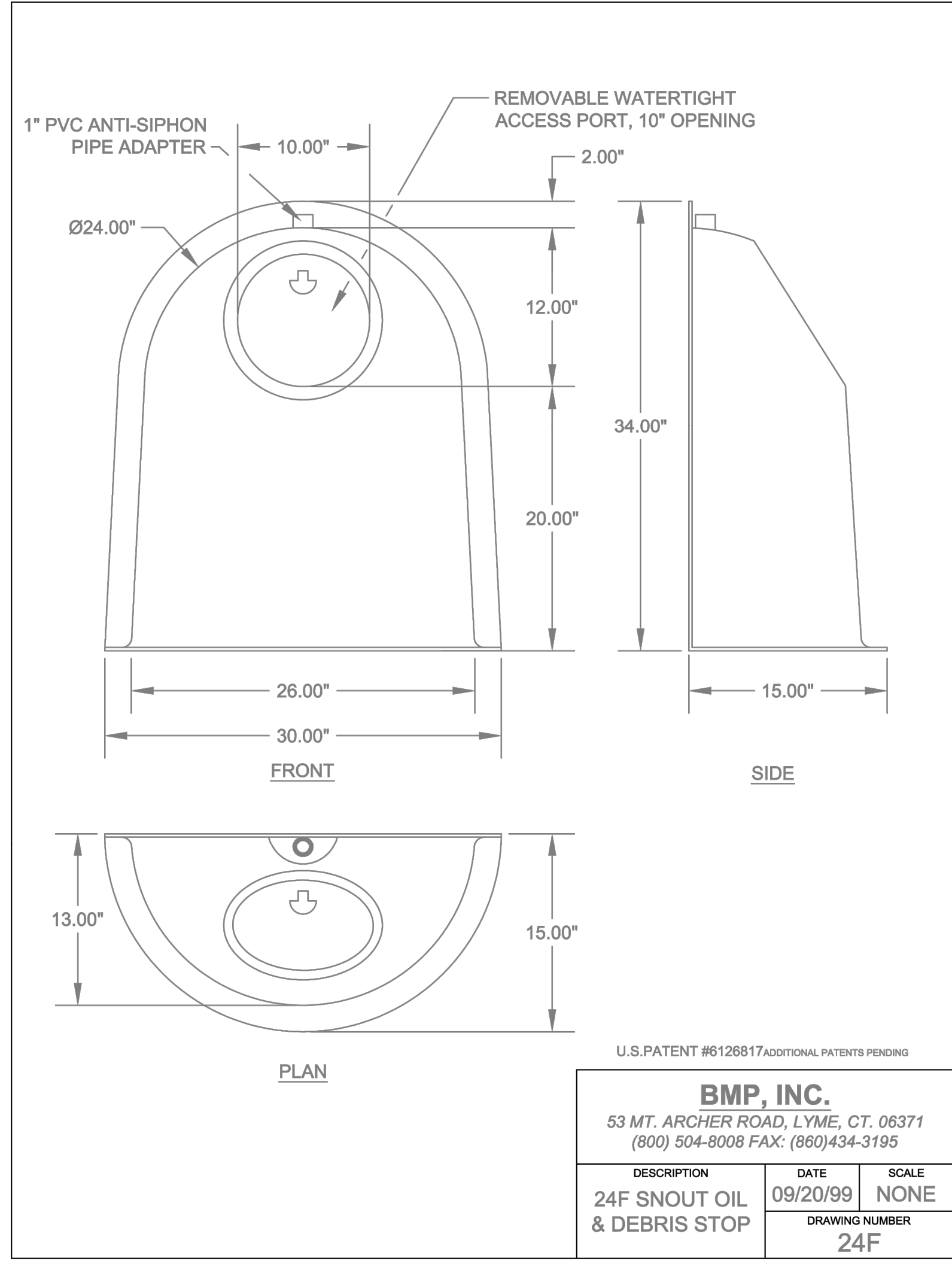
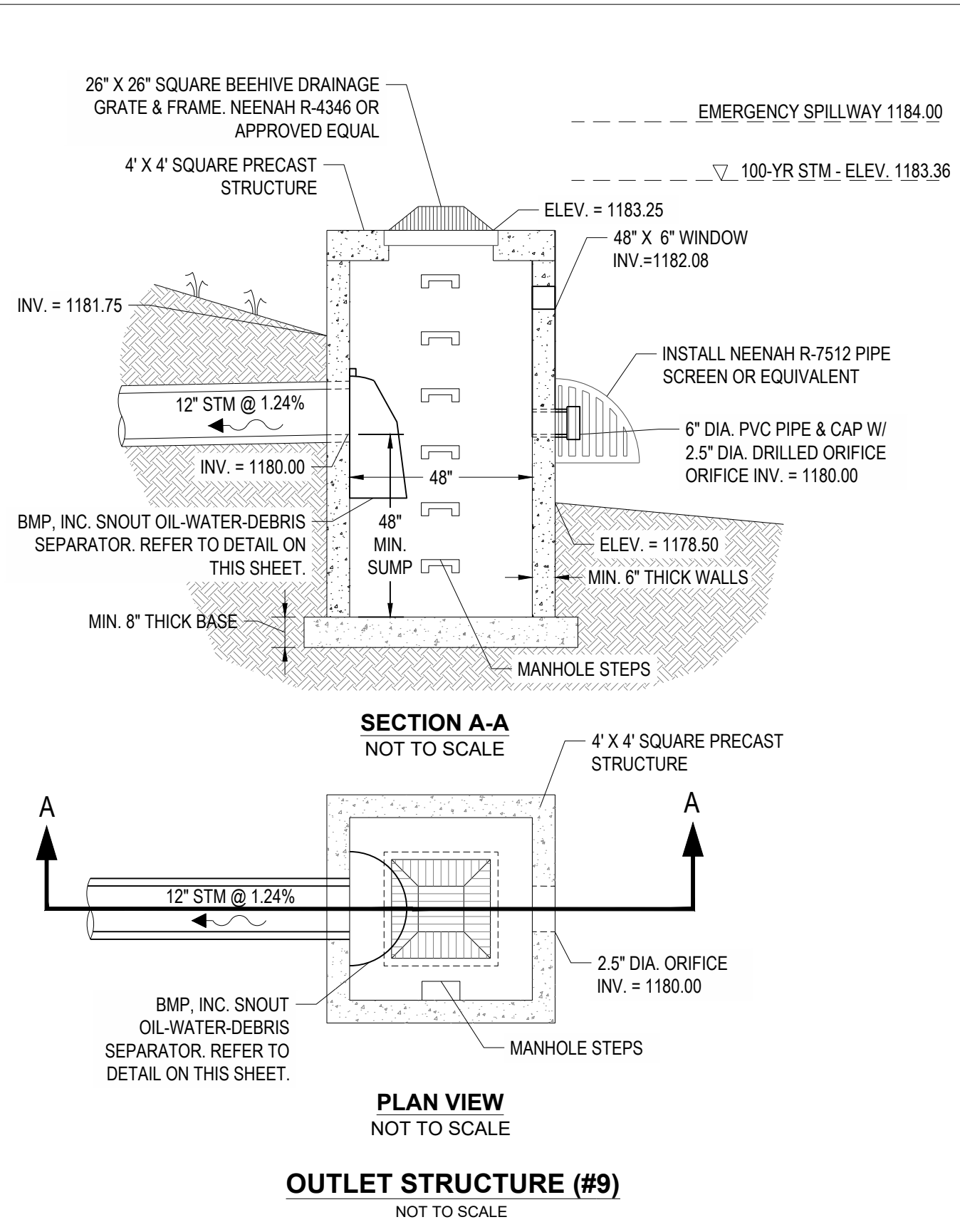
Issue: OUT TO BID

Drawing Title: **DETENTION DETAILS EAST**

C7.9

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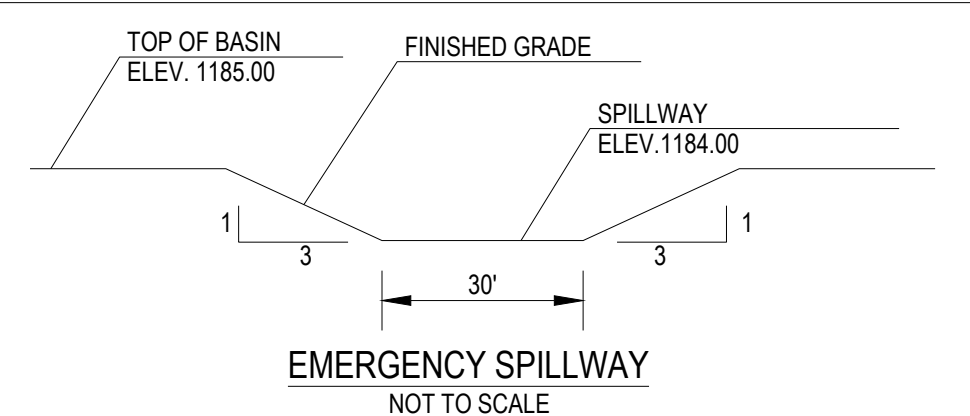
LEGEND

EXISTING

- MAJOR CONTOUR
- MINOR CONTOUR
- - - RIDGE LINE
- STM
- STORM SEWER
- STORM CURB INLET
- STORM HALF-HEIGHT HEADWALL
- XXX.XX X SPOT ELEVATION
- XXX.XX XXX.XX TOP OF CURB
- XXX.XX TOP OF CASTING ELEVATION
- STANDARD FLEXAMAT

PROPOSED

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West Basin Spillway Calculations

(Q100) Estimated 100-yr Flow = **77.47** cfs

(Qs) Spillway Capacity = $C_w \cdot L \cdot (h^{3/2})$

Spillway Coefficient, $C_w = 2.90$

Bottom Width, $L = 30.00$ ft

Height, $h = 1.00$ ft

$Q_s = 87.00$ cfs

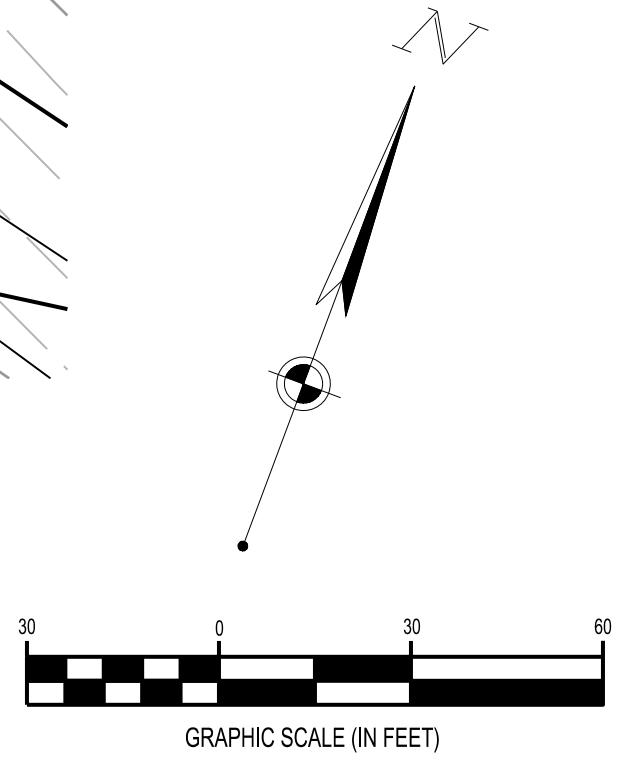
West Dry Detention Pond Volume

Elevation	Contour Area (SF)	Incremental Volume (CF)	Total Volume (CF)
1178	1,088	0	0
1180	2,005	3,047	3,047
1181	17,394	8,435	11,482
1182	43,937	29,659	41,141
1183	55,013	49,371	90,512
1184	61,743	58,346	148,858
1185	65,522	63,623	212,481

West Drainage Area Stormwater Release Rate Summary

Storm Frequency	Pre-Developed Peak Flow (CFS)	Dry Detention Basin Peak Elevation	Dry Detention Basin Peak Flow (CFS)	Total Post-Developed Peak Flow (CFS)
2	2.98	1182.17	0.59	1.19
5	8.15	1182.33	1.83	2.42
10	13.28	1182.49	3.57	4.02
25	21.68	1182.77	5.70	8.72
50	29.08	1183.06	6.05	12.04
100	37.45	1183.36	6.39	14.19

STORMWATER CALCULATIONS BASED ON ODOT REGULATIONS. THE POST-DEVELOPED PEAK RUNOFF RATE FOR THE 2-YR. THROUGH 100-YR RETURN PERIOD STORM MUST NOT EXCEED THE PRE-DEVELOPED PEAK RUNOFF RATE FOR THE EQUIVALENT STORM RETURN PERIOD.



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LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

ID	Description	Date

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Project Number: 759267

Scale: 1" = 30'

Drawn By: FAR

Checked By: JTK

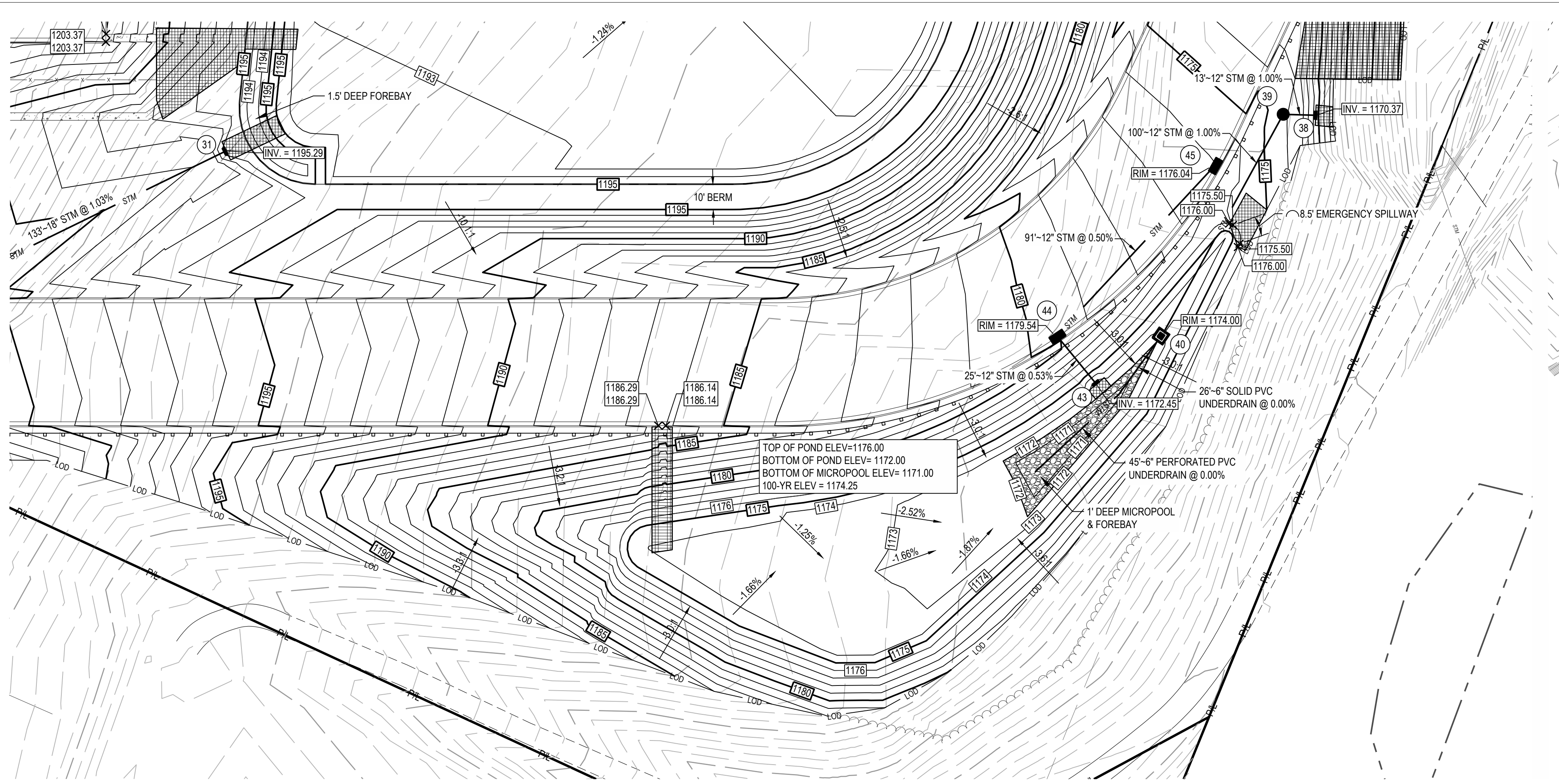
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Issue: OUT TO BID

Drawing Title:

DETENTION DETAILS WEST

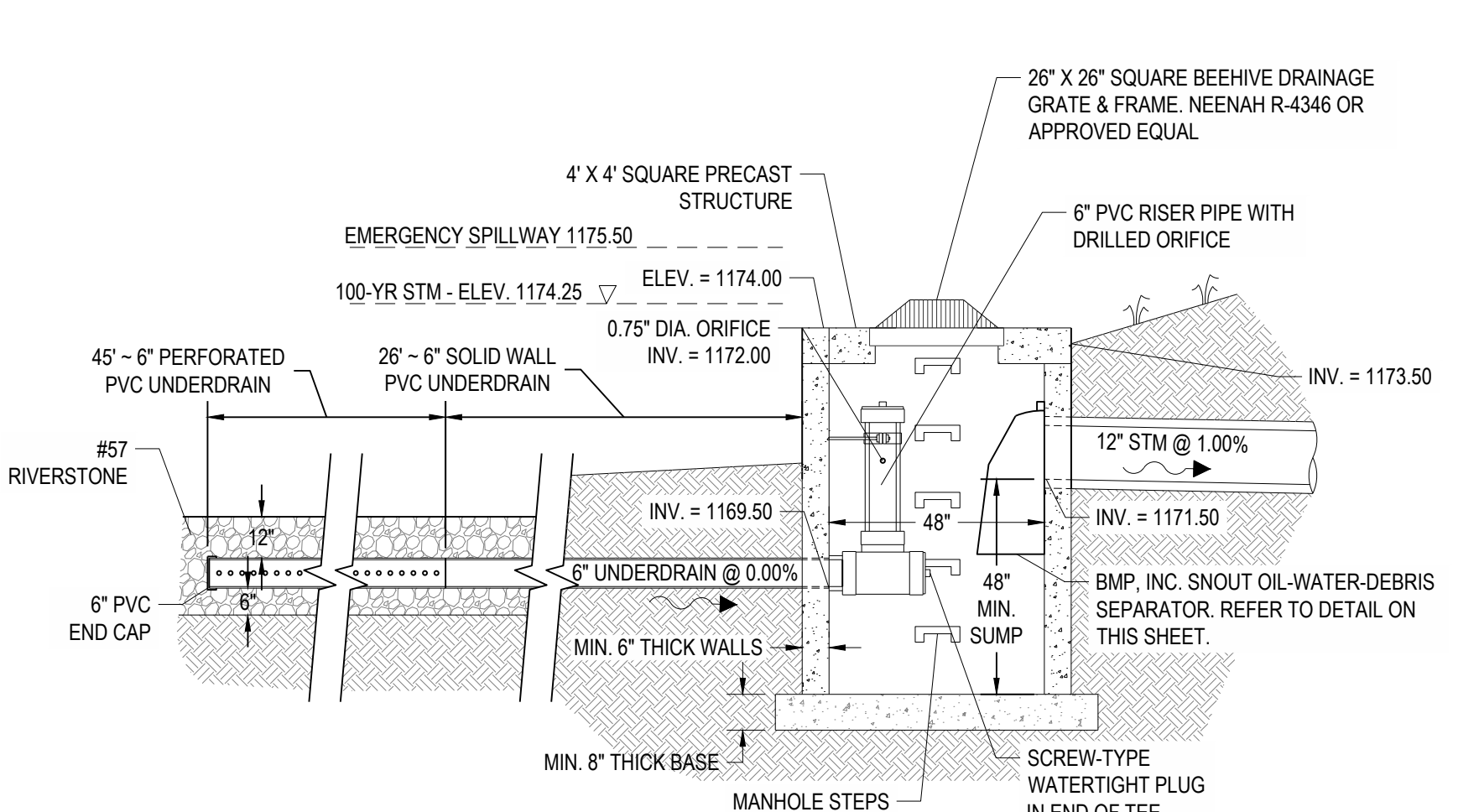
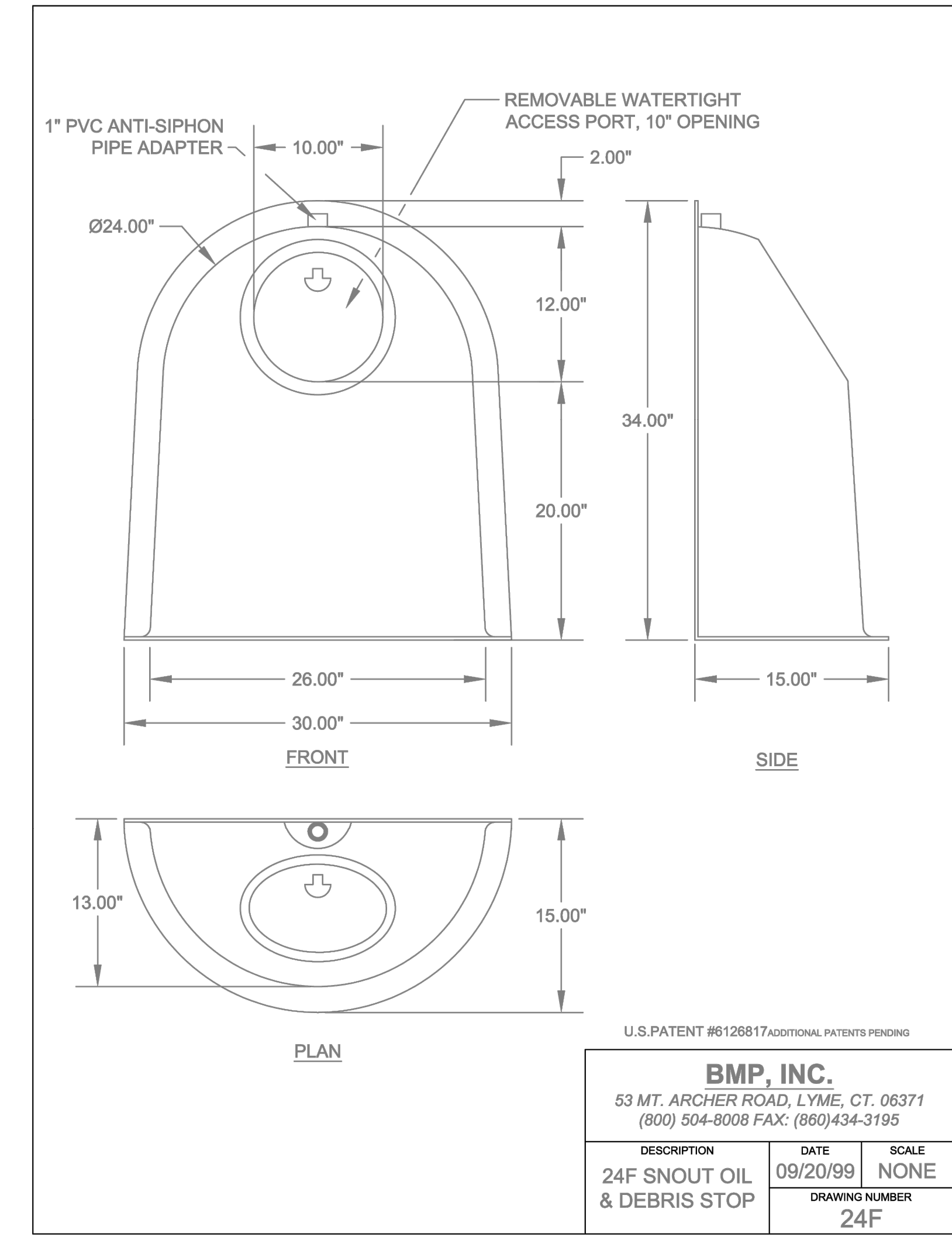
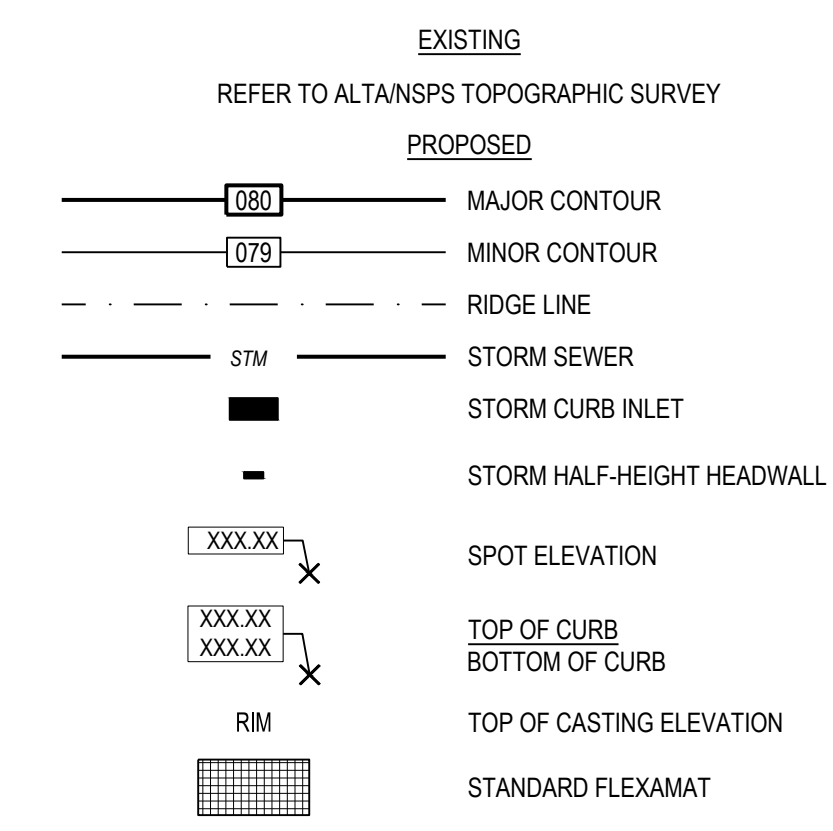
C7.10



GENERAL NOTES

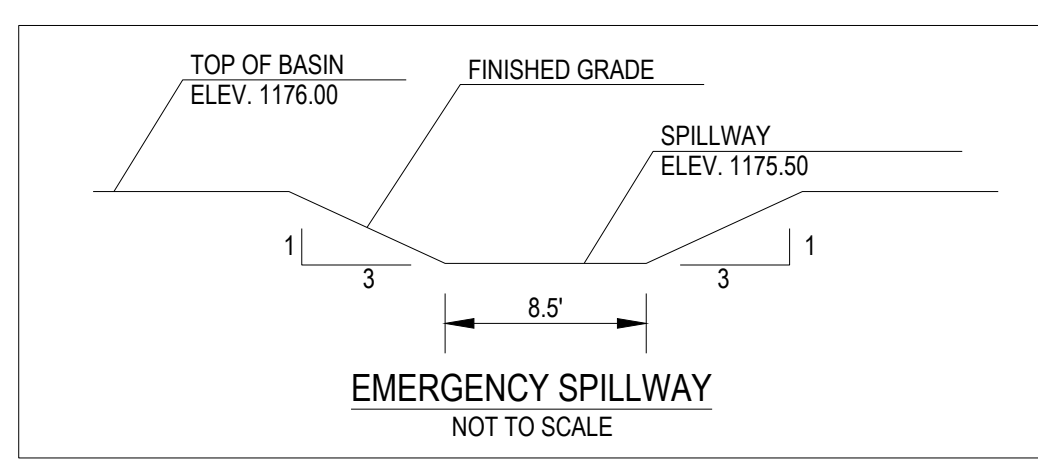
- ALL WORK AND MATERIALS SHALL COMPLY WITH ALL BELMONT COUNTY AND ODOT REGULATIONS AND CODES, AND O.S.H.A. STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL PERMITTING AND APPROVAL/INSPECTIONS AS REQUIRED FROM THE LOCALITY.
- STORM SEWER CATCH BASINS, CURB INLETS, MANHOLES AND ENDWALLS SHALL CONFORM TO ODOT STANDARD CONSTRUCTION DRAWINGS.
- REFER TO SWPPP PLANS FOR RIPRAP / FLEXAMAT LOCATIONS AND DIMENSIONS AND ALL OTHER PERMANENT EROSION CONTROL MEASURES.

LEGEND



Elevation	Contour Area (SF)	Incremental Volume (CF)	Total Volume (CF)
1171	417	0	0
1172	921	653	653
1173	3,458	2,055	2,708
1174	8,518	5,801	8,509
1175	10,886	9,678	18,187
1176	12,770	11,815	30,002

Storm Frequency	Pre-Developed Peak Flow (CFS)	Dry Detention Basin Peak Elevation	Dry Detention Basin Peak Flow (CFS)	Total Post-Developed Peak Flow (CFS)
2	0.95	1173.84	0.02	0.02
5	1.74	1174.01	0.07	0.07
10	2.46	1174.03	0.20	0.20
25	3.57	1174.08	0.71	0.71
50	4.52	1174.17	1.93	1.93
100	5.57	1174.25	3.59	3.59



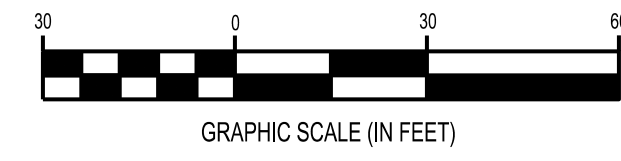
(Q100) Estimated 100-yr Flow =	8.45 cfs
(Qs) Spillway Capacity = $C_w \cdot L \cdot (h^{3/2})$	
Spillway Coefficient, C_w =	2.90
Bottom Width, L =	8.50 ft
Height, h =	0.50 ft
Q_s =	8.72 cfs

STORMWATER CALCULATIONS BASED ON ODOT REGULATIONS. THE POST-DEVELOPED PEAK RUNOFF RATE FOR THE 2-YR. THROUGH 100-YR RETURN PERIOD STORM MUST NOT EXCEED THE PRE-DEVELOPED PEAK RUNOFF RATE FOR THE EQUIVALENT STORM RETURN PERIOD.

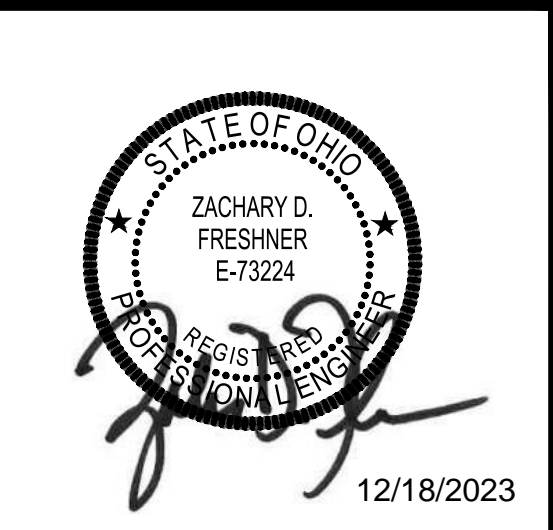
BMP WATER QUALITY STRUCTURE NOTICE

THE OUTLET STRUCTURE IS AN INTEGRAL PART OF THE PRIVATE STORM SEWER SYSTEM DEPICTED IN THESE DRAWINGS. RESPONSIBILITY AND ASSURANCE OF PERIODIC MAINTENANCE AND THE CONTINUOUS FUNCTIONALITY OF THESE STORMWATER QUALITY DEVICES IS PERPETUAL, BEGINNING WITH THE OWNER AT THE TIME OF INSTALLATION AND CONTINUING TO ALL FUTURE OWNERS OF SAID PRIVATE STORM SEWER SYSTEM.

CONTRACTOR TO ENSURE THAT THE DETENTION SYSTEM (INCLUDING ALL DETENTION STRUCTURES AND WATER QUALITY STRUCTURES) IS CLEAN AND FUNCTIONS PROPERLY BEFORE SYSTEM IS TURNED OVER TO THE OWNER.



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

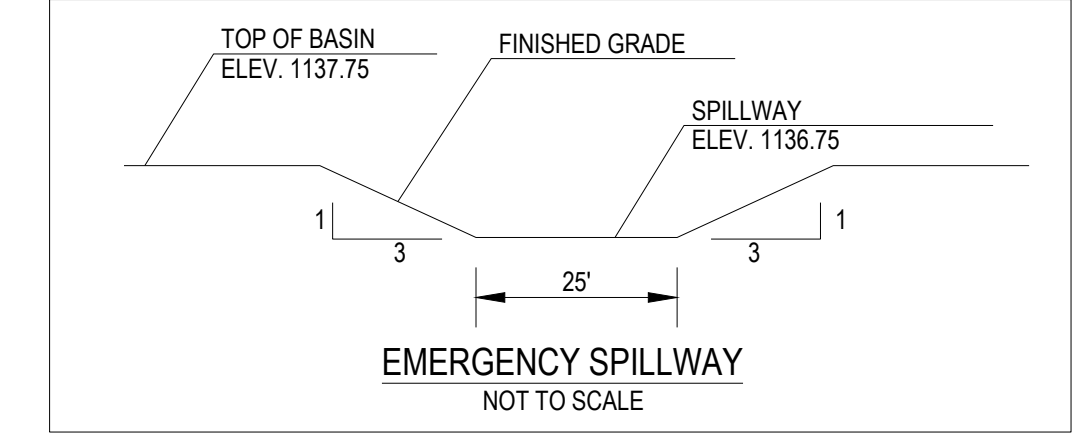
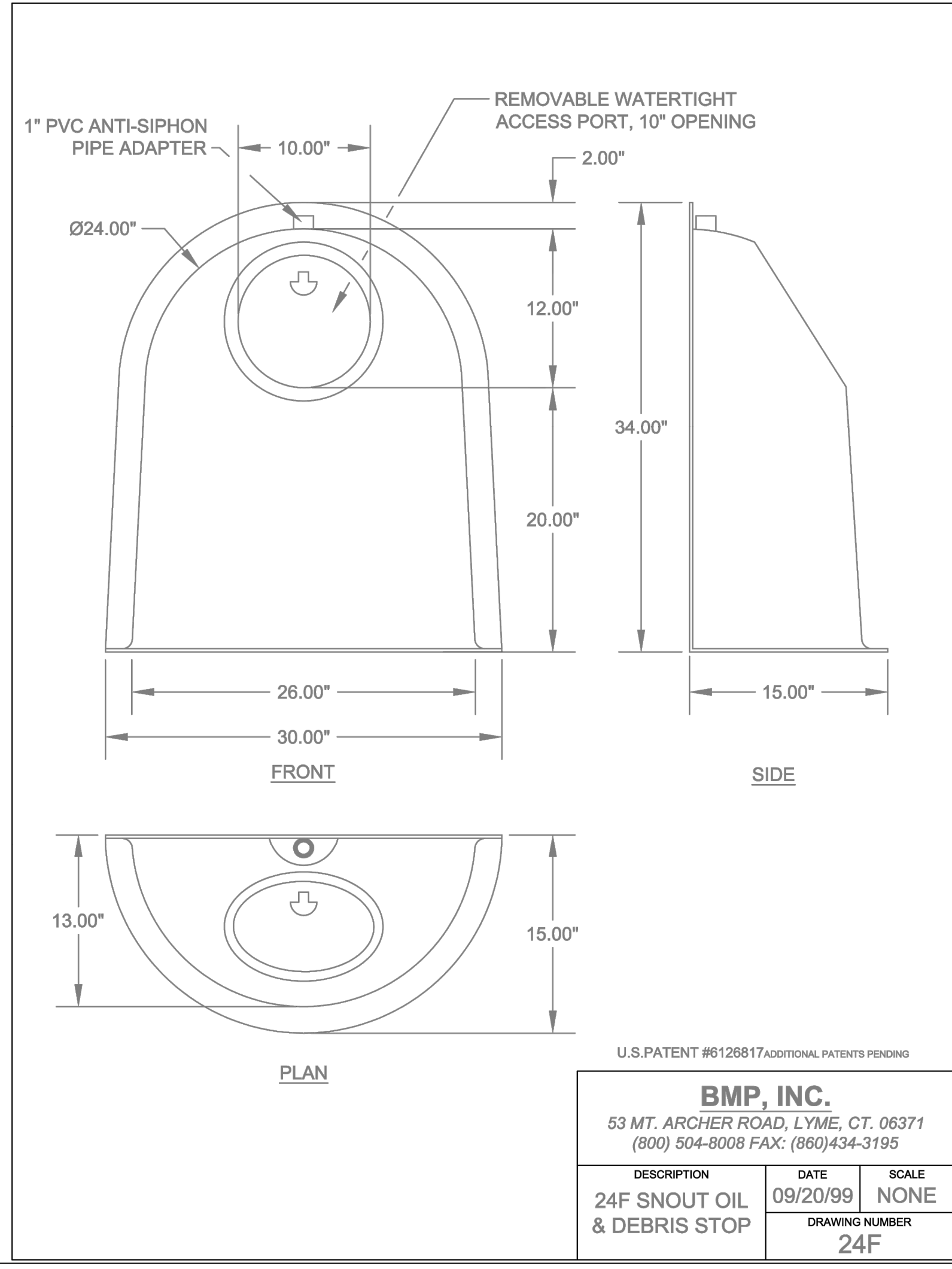
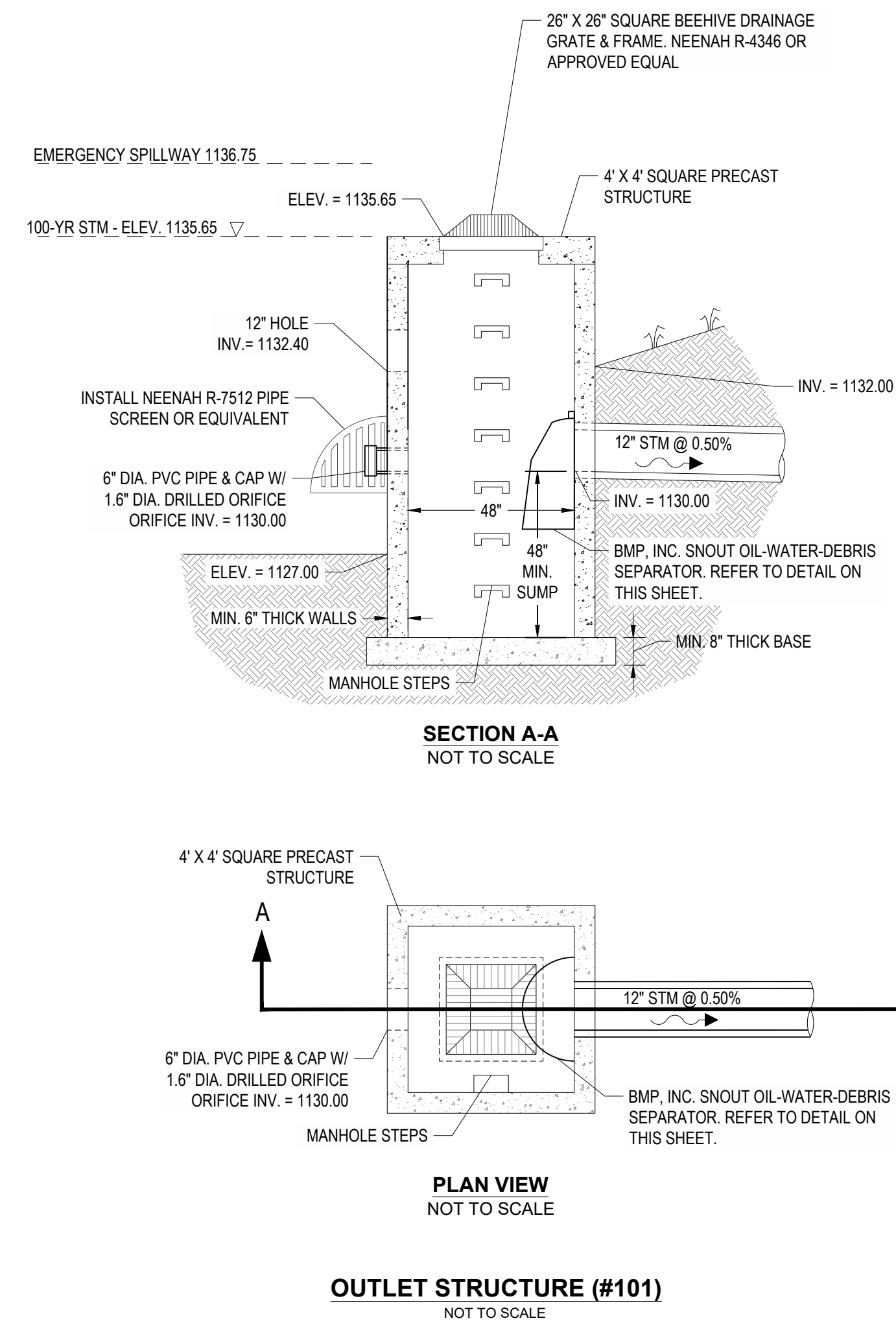
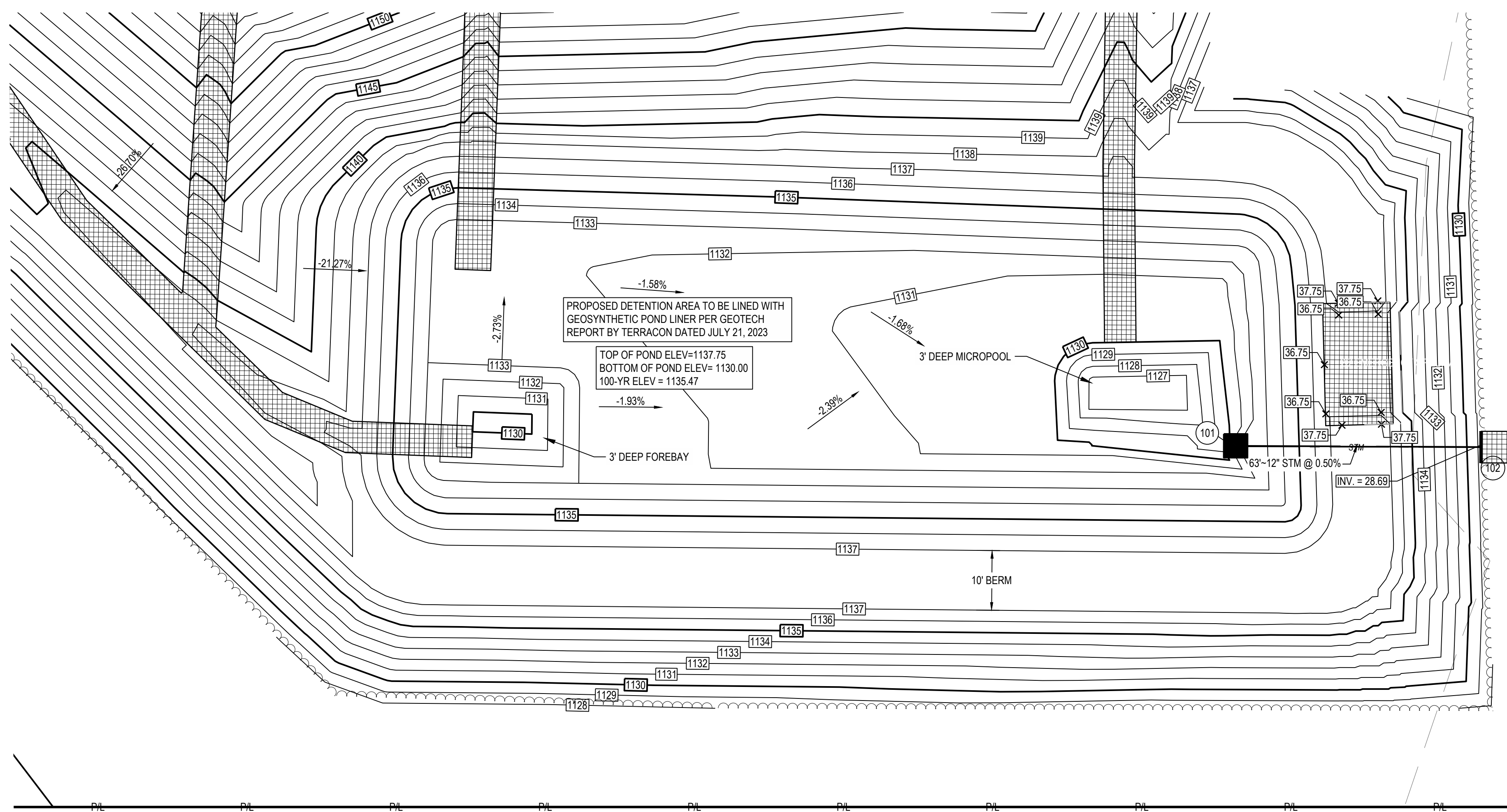
Revisions / Submissions		
ID	Description	Date

Drawing Title: **DETENTION DETAILS SOUTHEAST**

Project Number: 759267
 Scale: 1" = 30'
 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

C7.11

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South Basin Spillway Calculations

(Q100) Estimated 100-yr Flow =	62.68	cfs
(Qs) Spillway Capacity = $C_w \cdot L \cdot (h^{3/2})$		
Spillway Coefficient, C_w =	2.90	
Bottom Width, L =	25.00	ft
Height, h =	1.00	ft
Qs =	72.50	cfs

STORMWATER CALCULATIONS BASED ON ODOT REGULATIONS. THE POST-DEVELOPED PEAK RUNOFF RATE FOR THE 2-YR. THROUGH 100-YR RETURN PERIOD STORM MUST NOT EXCEED THE PRE-DEVELOPED PEAK RUNOFF RATE FOR THE EQUIVALENT STORM RETURN PERIOD.

PRE-DEVELOPMENT 2-YR PEAK RUNOFF RATE: 6.62 CFS
 POST-DEVELOPMENT 2-YR PEAK RUNOFF RATE: 3.92 CFS
 PRE-DEVELOPMENT 100-YR PEAK RUNOFF RATE: 43.57 CFS
 POST-DEVELOPMENT 100-YR PEAK RUNOFF RATE: 22.02 CFS
 STORAGE VOLUME PROVIDED: 87,507 CF FROM ELEVATION 1130.00 TO 1136.75

- GENERAL NOTES**
- ALL WORK AND MATERIALS SHALL COMPLY WITH ALL BELMONT COUNTY AND ODOT REGULATIONS AND CODES, AND O.S.H.A. STANDARDS. THE CONTRACTOR SHALL OBTAIN FINAL PERMITTING AND APPROVAL/INSPECTIONS AS REQUIRED FROM THE LOCALITY.
 - STORM SEWER CATCH BASINS, CURB INLETS, MANHOLES AND ENDWALLS SHALL CONFORM TO ODOT STANDARD CONSTRUCTION DRAWINGS.
 - REFER TO SWPPP PLANS FOR RIPRAP LOCATIONS AND DIMENSIONS AND ALL OTHER PERMANENT EROSION CONTROL MEASURES.

South Drainage Area Stormwater Release Rate Summary

Storm Frequency	Pre Developed Release Rates (CFS)	Post Developed Release Rates (CFS)	Dry Detention Basin Peak Flow (CFS)	Water Elevation in Dry Detention Basin	Total Post-Developed Peak Flow (CFS)
2	6.62	3.92	1.23	1132.95	4.05
5	12.91	7.26	2.86	1133.42	6.65
10	18.59	10.63	3.91	1133.90	9.68
25	27.49	15.13	5.02	1134.57	14.38
50	35.10	18.51	5.77	1135.11	17.85
100	43.57	22.02	6.44	1135.65	21.31

BMP WATER QUALITY STRUCTURE NOTICE

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CONTRACTOR TO ENSURE THAT THE DETENTION SYSTEM (INCLUDING ALL DETENTION STRUCTURES AND WATER QUALITY STRUCTURES) IS CLEAN AND FUNCTIONS PROPERLY BEFORE SYSTEM IS TURNED OVER TO THE OWNER.

DISCLAIMER: PROPOSED BASIN HAS BEEN DESIGNED TO HANDLE 3.42 ACRES (148,975.20 S.F.) OF IMPERVIOUS AREA.



STATE OF OHIO
 ZACHARY D. FRESHNER
 E-79224
 REGISTERED PROFESSIONAL ENGINEER
 12/18/2023

LOVES TRAVEL STOP
 ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

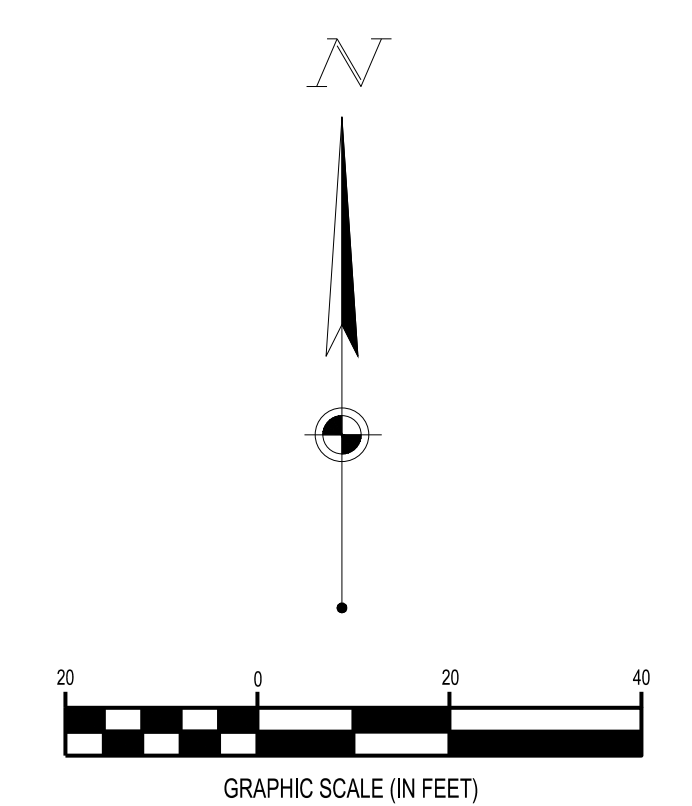
Revisions / Submissions

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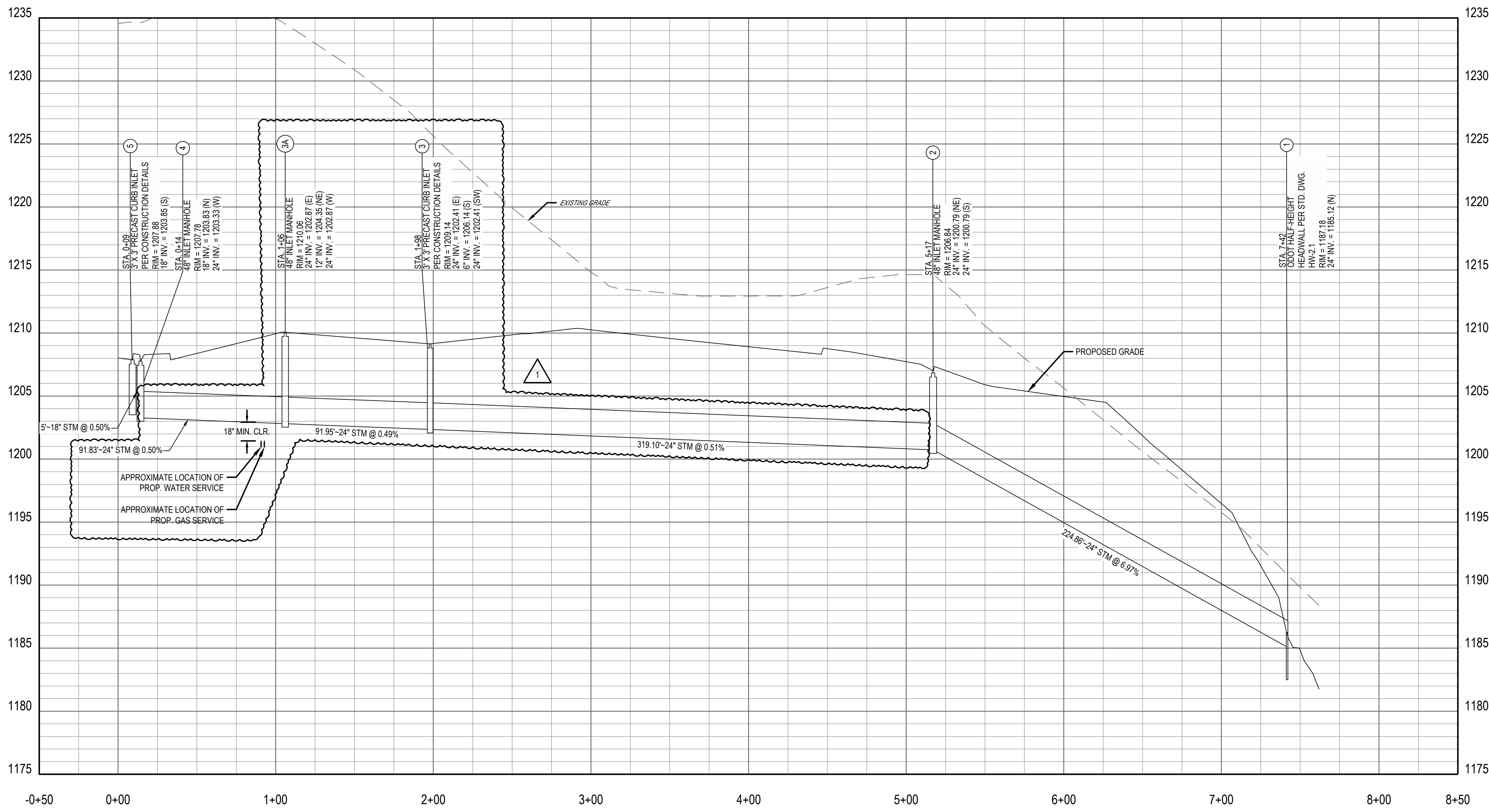
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 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
**DETENTION DETAILS
 PREPARED PAD**

C7.12



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LEGEND
 - - - - - EXISTING GRADE
 _____ PROPOSED GRADE

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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STATE OF OHIO
 ZACHARY D. FRESHNER
 E-73224
 REGISTERED PROFESSIONAL ENGINEER
 01/15/2024

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

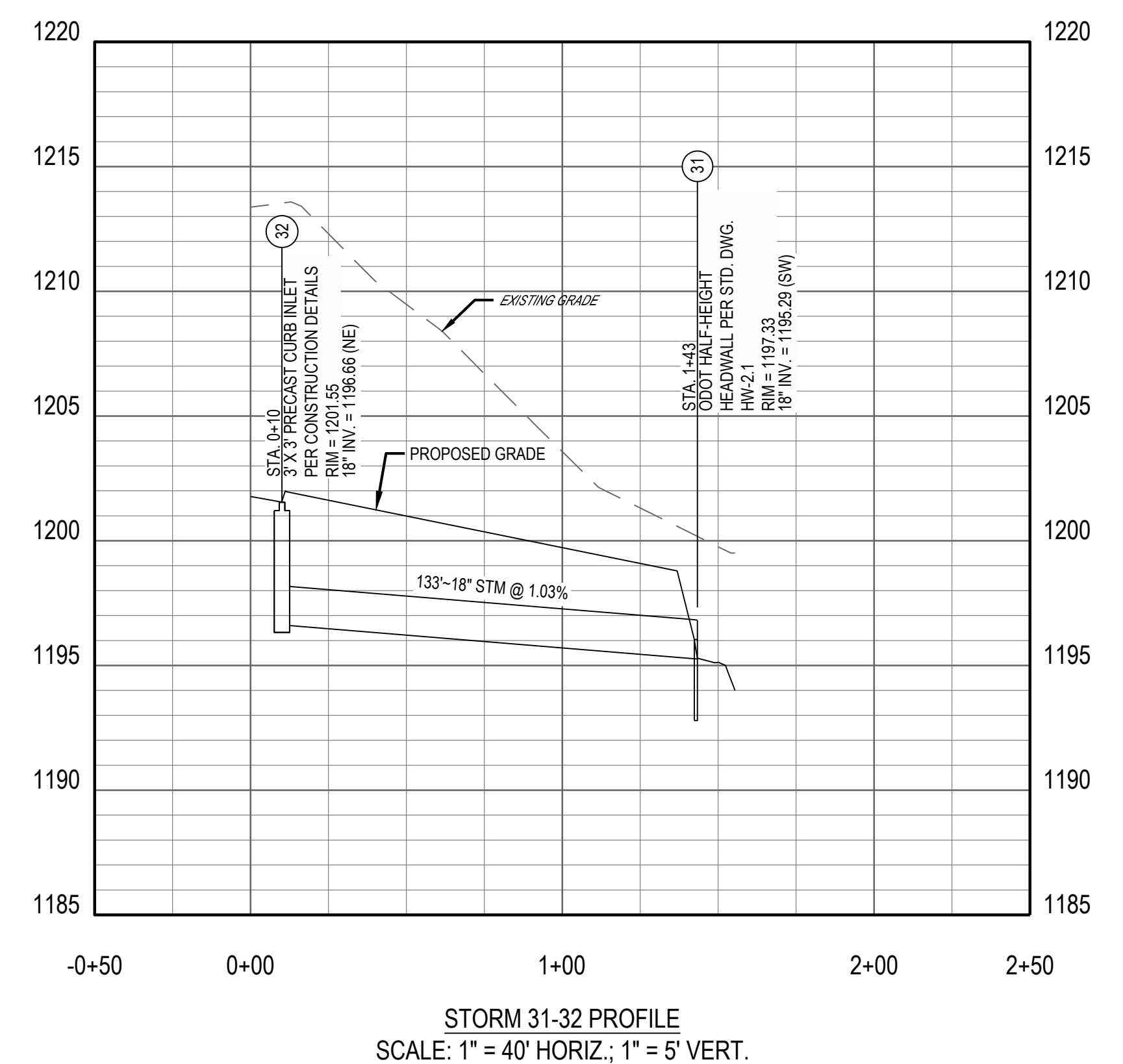
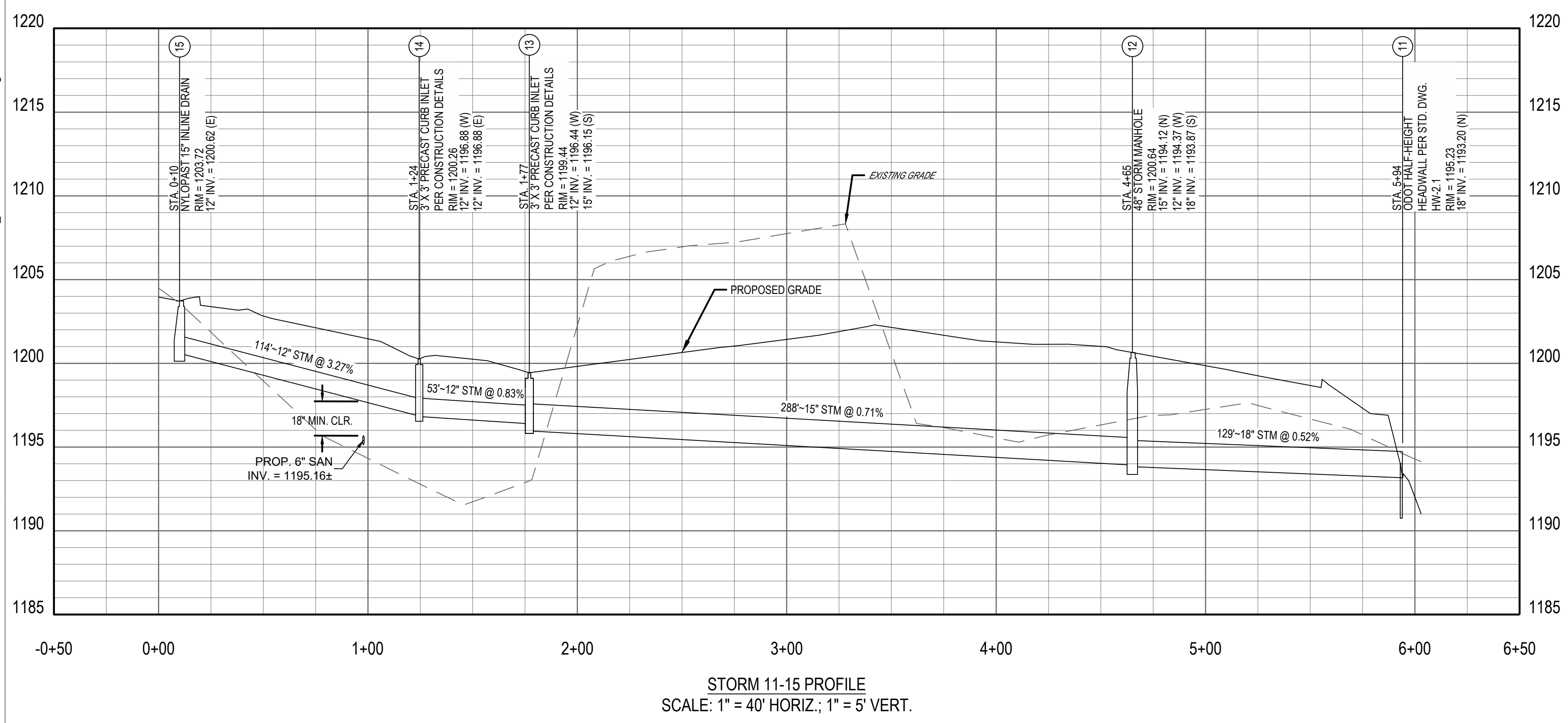
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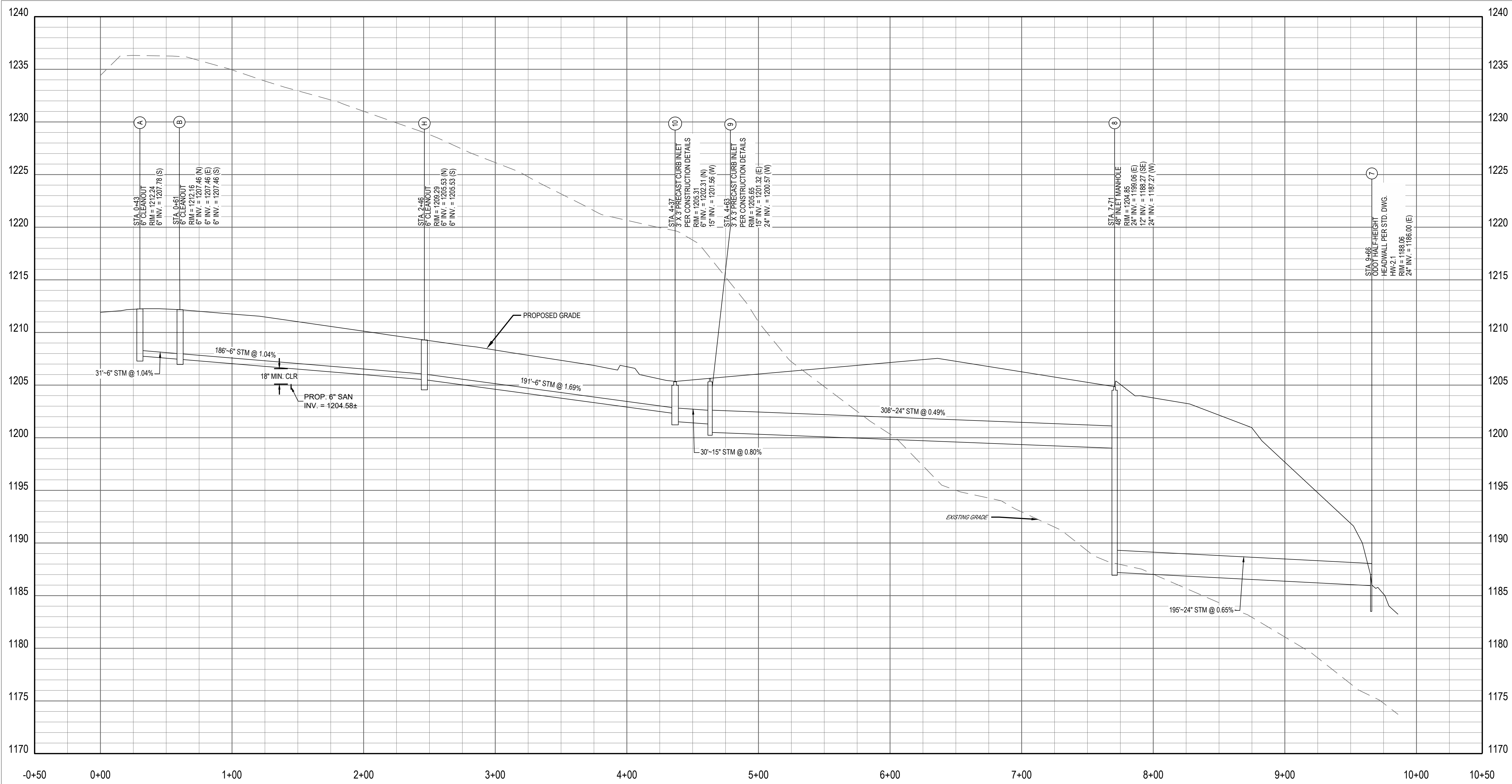
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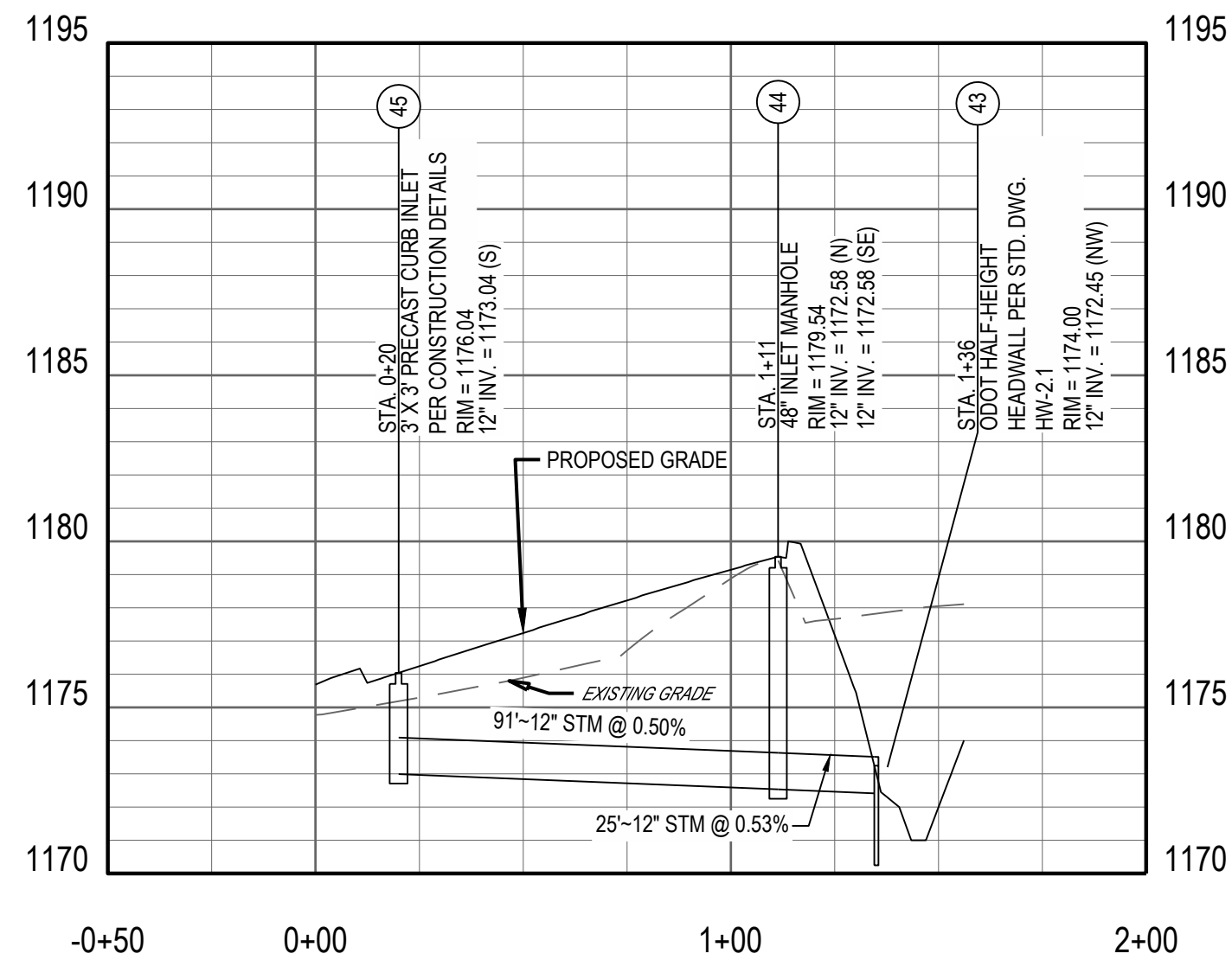
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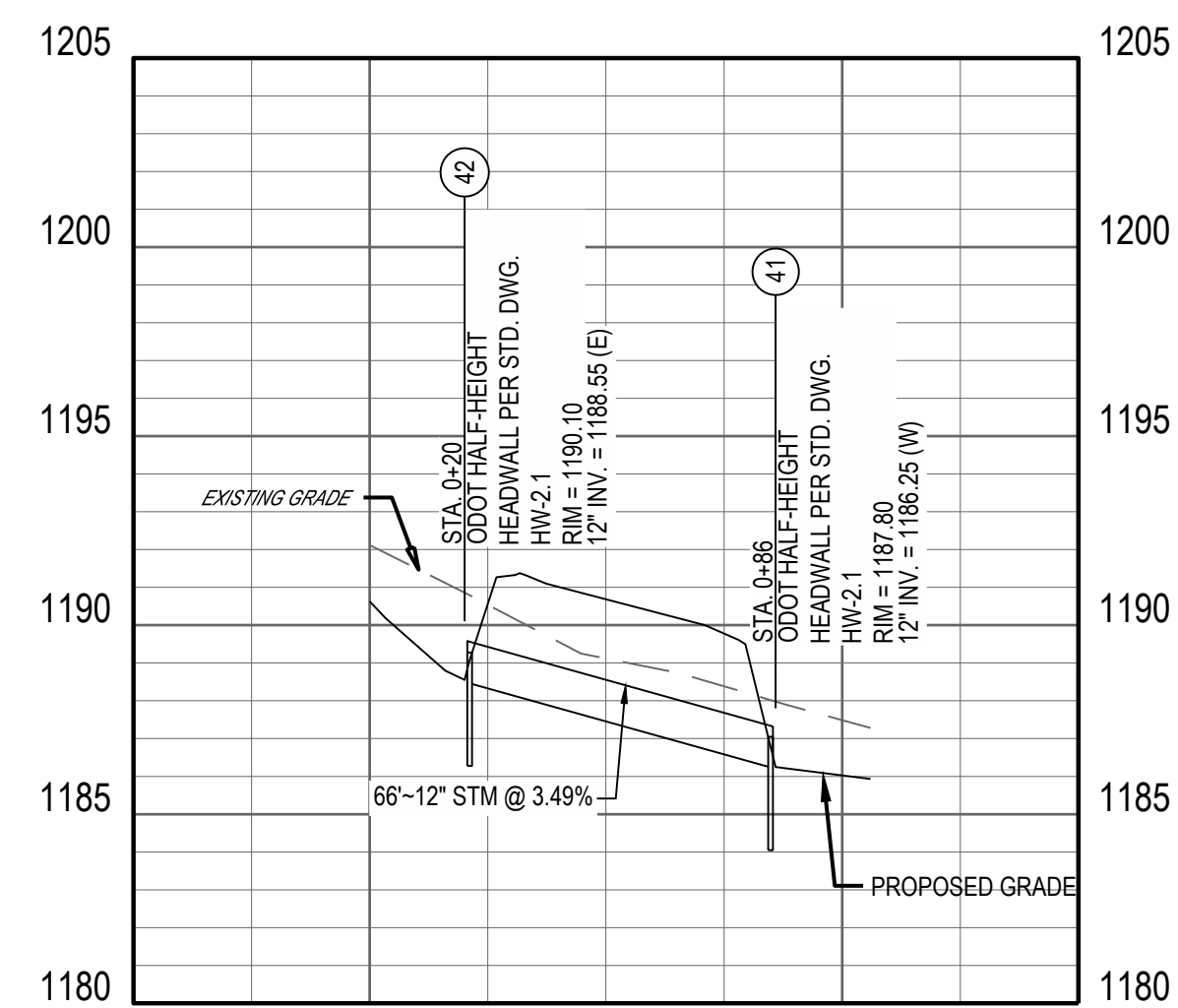
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STORM 7-A PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.



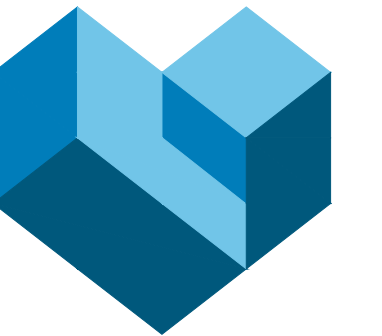
STORM 43-45 PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.



STORM 41-42 PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.

LEGEND
 - - - - - EXISTING GRADE
 _____ PROPOSED GRADE

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Akron, OH 44321
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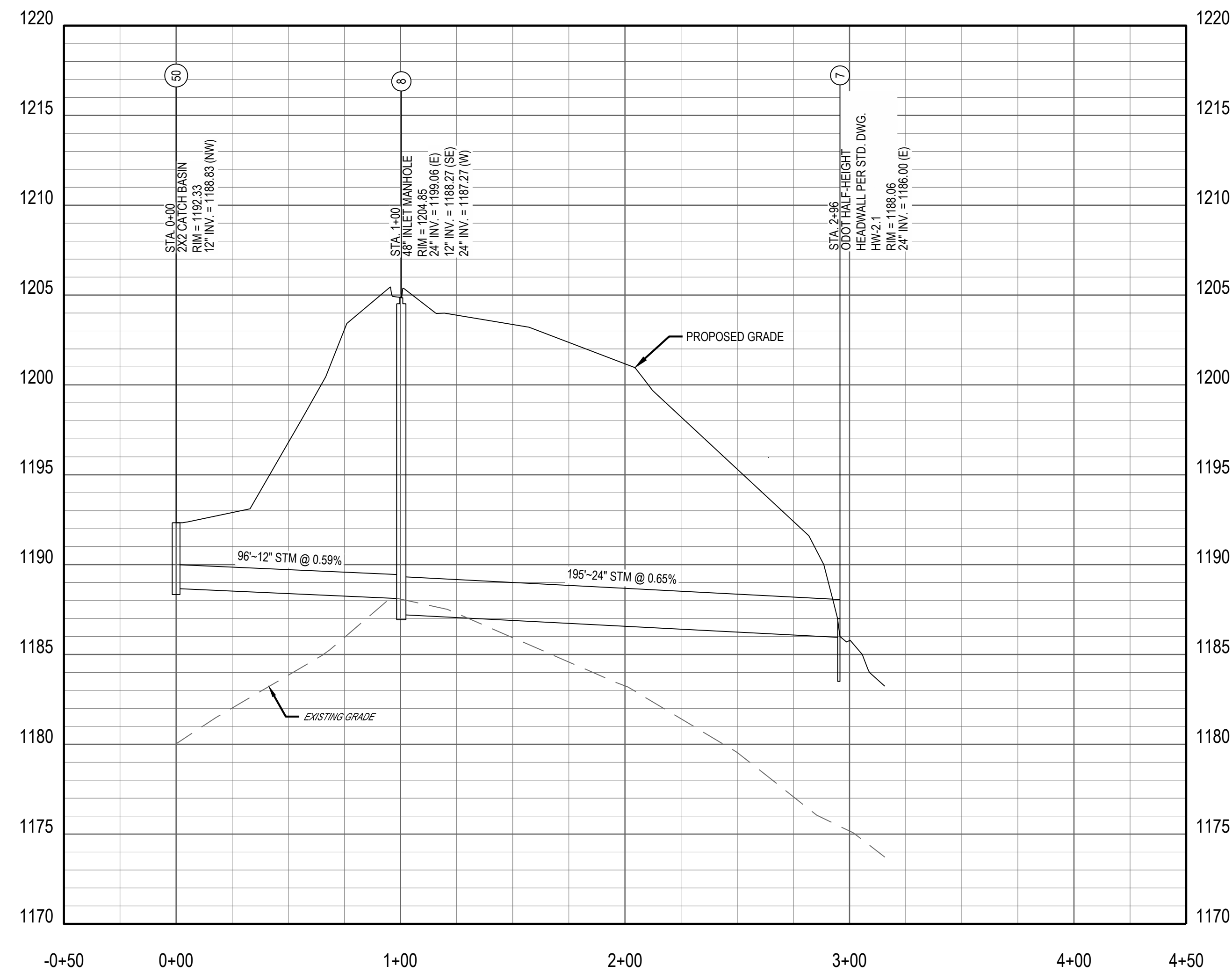
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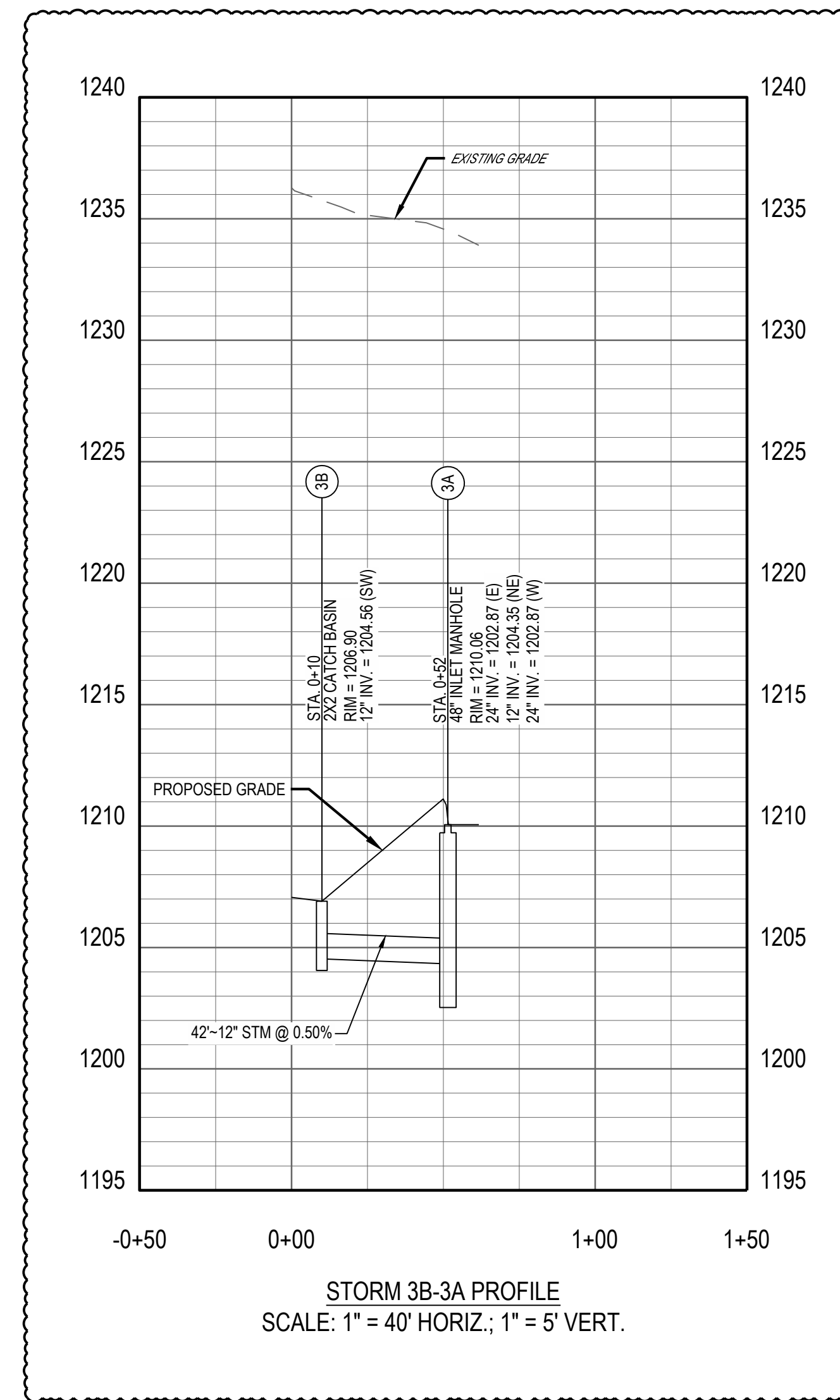
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 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
STORM PROFILE

C7.14

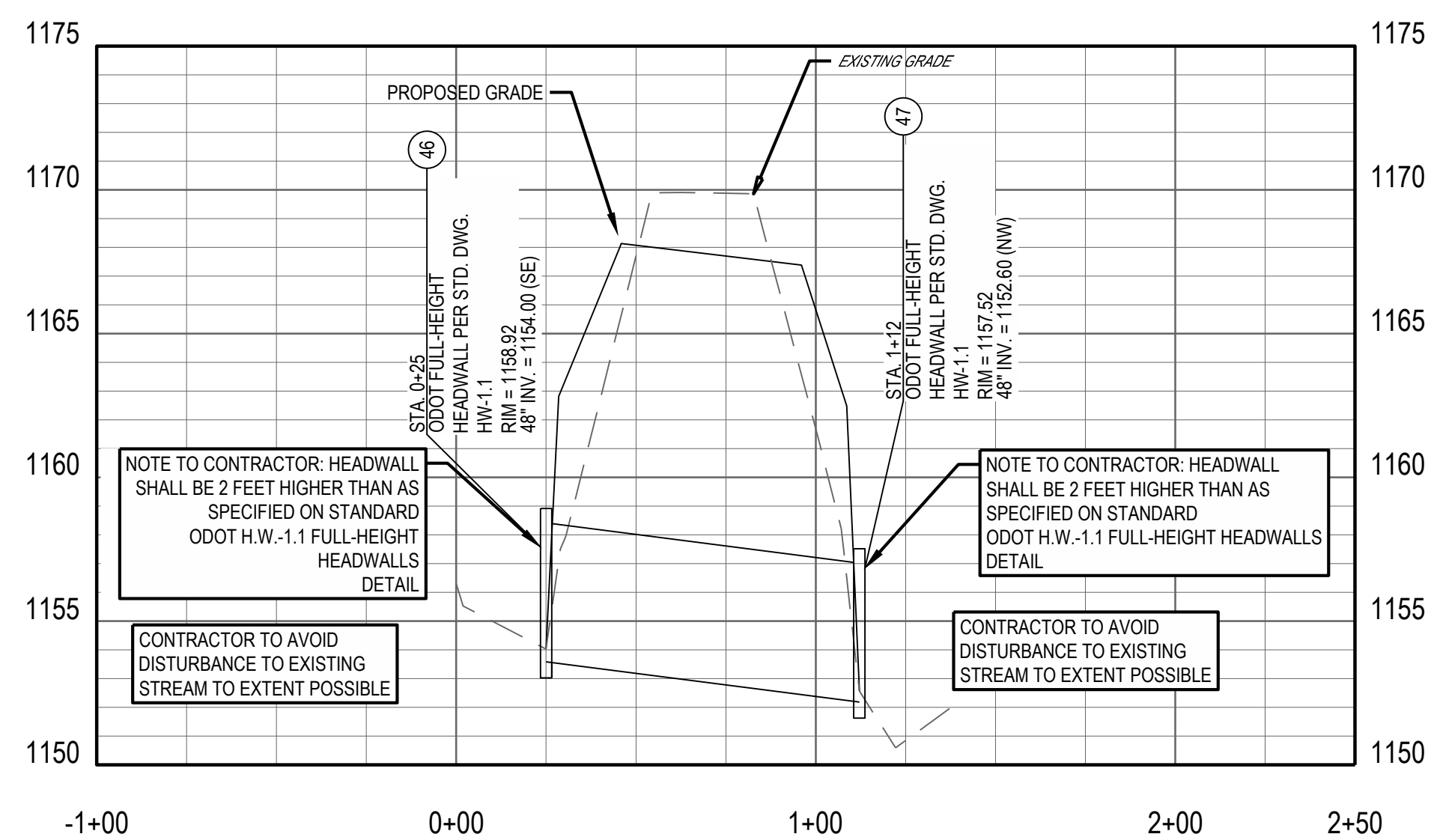


STORM 111-8-7 PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.



STORM 3B-3A PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.

LEGEND
 - - - - - EXISTING GRADE
 ——— PROPOSED GRADE



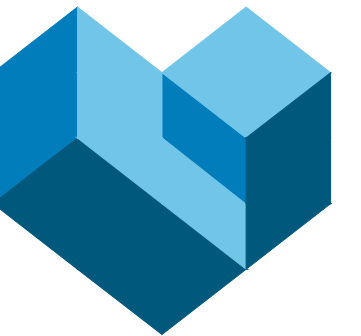
STORM 45-46 PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.

NOTE TO CONTRACTOR: HEADWALL SHALL BE 2 FEET HIGHER THAN AS SPECIFIED ON STANDARD ODOT H.W.-1.1 FULL-HEIGHT HEADWALLS DETAIL

NOTE TO CONTRACTOR: HEADWALL SHALL BE 2 FEET HIGHER THAN AS SPECIFIED ON STANDARD ODOT H.W.-1.1 FULL-HEIGHT HEADWALLS DETAIL

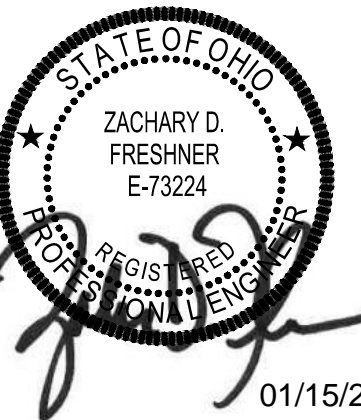
CONTRACTOR TO AVOID DISTURBANCE TO EXISTING STREAM TO EXTENT POSSIBLE

CONTRACTOR TO AVOID DISTURBANCE TO EXISTING STREAM TO EXTENT POSSIBLE



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LOVES TRAVEL STOP

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Revisions / Submissions		
ID	Description	Date
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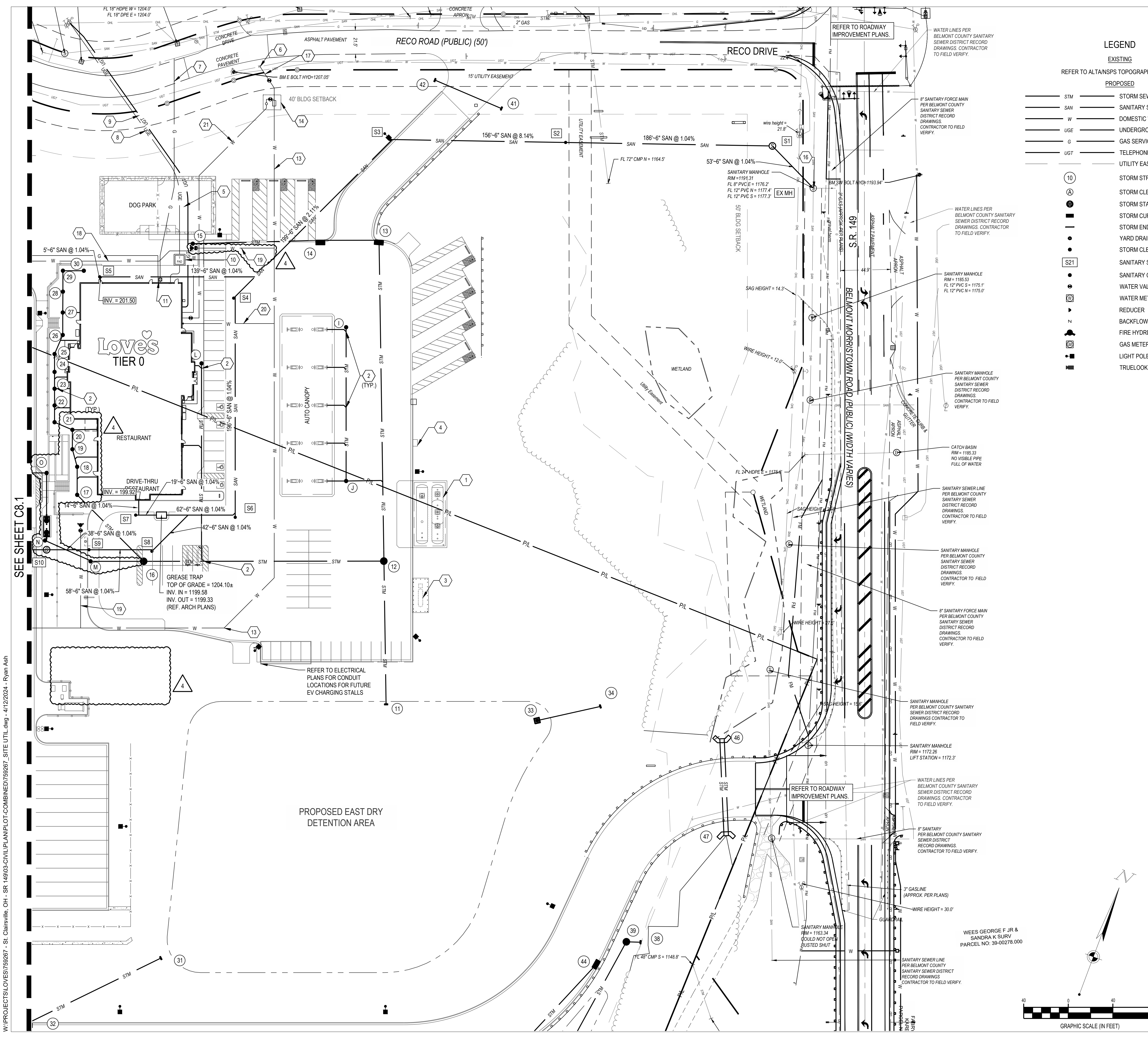
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 Project Number: 759267
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 Drawn By: FAR
 Checked By: JTK
 Date: 12/19/2023
 Issue: OUT TO BID

Drawing Title:
STORM PROFILE

C7.15



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



CODED NOTES:

- UNDERGROUND STORAGE TANKS. REFERENCE ARCHITECTURAL PLANS AND FUELING PLANS.
- WYE CONNECTION
- PROPANE STORAGE AREA. REFERENCE ARCHITECTURAL PLANS.
- AIR/ WATER ISLAND. REFERENCE ARCHITECTURAL PLANS.
- CONTRACTOR SHALL PROVIDE (2) - 4" CONDUITS TO BUILDING FOR TELEPHONE AND COMMUNICATIONS. COORDINATE WITH BUILDING CONTRACTOR AND TELEPHONE/COMMUNICATIONS COMPANY FOR CONNECTION AND EXACT LOCATION OF PROPOSED SERVICE.
- CONNECT TO EXISTING WATERMAIN. CONTRACTOR TO COORDINATE CONNECTION WITH BELMONT COUNTY WATER AND SEWER DISTRICT. REFER TO SHEET C9.4 FOR TIE-IN DETAILS.
- PROPOSED NATURAL GAS SERVICE. CONTRACTOR TO COORDINATE WITH GAS PROVIDER FOR FINAL LOCATION TO GAS METER AND CONNECTION.
- PROPOSED UNDERGROUND ELECTRIC SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH ELECTRIC COMPANY.
- PROPOSED UNDERGROUND TELEPHONE SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH TELEPHONE COMPANY.
- APPROXIMATE LOCATION OF PROPOSED TRANSFORMER PAD.
- APPROXIMATE LOCATION OF GAS METER.
- PROPOSED WATER SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH WATER COMPANY.
- PROPOSED 8" PRIVATE FIRE LINE SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH WATER COMPANY.
- PROPOSED 2" METER VAULT FOR BACK FLOW PREVENTOR AND METER PER BELMONT COUNTY WATER AND SEWER DISTRICT DETAIL. REFER TO SHEET C9.4.
- PROPOSED FIRE PUMP HOUSE. REFER TO DETAIL SHEET C9.7 AND DESIGN BY FLUID SOLUTIONS.
- CORE DRILL EXISTING MANHOLE AND USE KOR-N-SEAL BOOT OR APPROVED EQUAL. CORE DRILL THRU OR MODIFY CHANNEL FOR FLOW.
- PROPOSED 8" PRIVATE WATER LINE.
- PROPOSED 1.25" PRIVATE WATER SERVICE LINE.
- PROPOSED 6" PRIVATE FIRE LINE SERVICE.
- PROPOSED 4" PRIVATE FIRE LINE SERVICE.
- PROPOSED 3" PRIVATE WATER SERVICE LINE.

LEGEND

- EXISTING**
REFER TO ALTANSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- STM STORM SEWER
 - SAN SANITARY SEWER
 - DW DOMESTIC WATER SERVICE
 - UG UNDERGROUND ELECTRIC SERVICE
 - G GAS SERVICE
 - UGT TELEPHONE SERVICE
 - UTILITY EASEMENT
- (10) STORM STRUCTURE NUMBER
 - (A) STORM CLEANOUT NUMBER
 - (S) STORM STANDARD MANHOLE
 - (S1) STORM CURB INLET
 - (S2) STORM ENDWALL
 - (S3) STORM CLEANOUT
 - (S4) SANITARY STRUCTURE NUMBER
 - (S5) SANITARY CLEANOUT
 - (S6) WATER VALVE
 - (S7) WATER METER
 - (S8) REDUCER
 - (S9) BACKFLOW PREVENTER
 - (S10) FIRE HYDRANT
 - (S11) GAS METER
 - (S12) LIGHT POLE
 - (S13) TRUELOOK CAMERA

SANITARY SEWER STRUCTURE SCHEDULE

NO.	STRUCTURE	RIM	INVERT
EX MH	EXISTING MANHOLE	1190.08	1177.30 (6") NW 1177.40 (12") N 1175.90 (12") S 1176.20 (8") E
S1	MANHOLE	1189.15	1177.85 (6") W 1177.85 (6") SE
S2	CLEANOUT	1185.57	1179.78 (6") W 1179.78 (6") E
S3	CLEANOUT	1196.44	1192.44 (6") SW 1192.44 (6") E
S4	CLEANOUT	1203.25	1196.65 (6") S 1196.65 (6") NE
S5	CLEANOUT	1204.96	1201.45 (6") SE 1197.57 (6") E
S6	CLEANOUT	1203.29	1198.69 (6") W 1198.69 (6") N
S7	CLEANOUT	1204.22	1199.78 (6") N 1199.78 (6") E
S8	CLEANOUT	1203.38	1199.57 (6") W 1199.57 (6") NE
S9	CLEANOUT	1203.67	1200.17 (6") W 1200.17 (6") E
S10	SANITARY DROP MANHOLE	1212.05	1202.15 (6") W 1200.56 (6") E



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions

ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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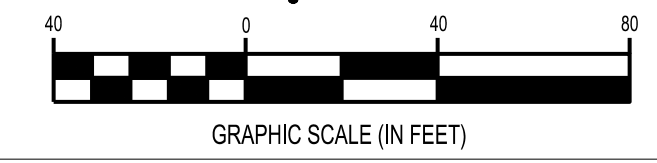
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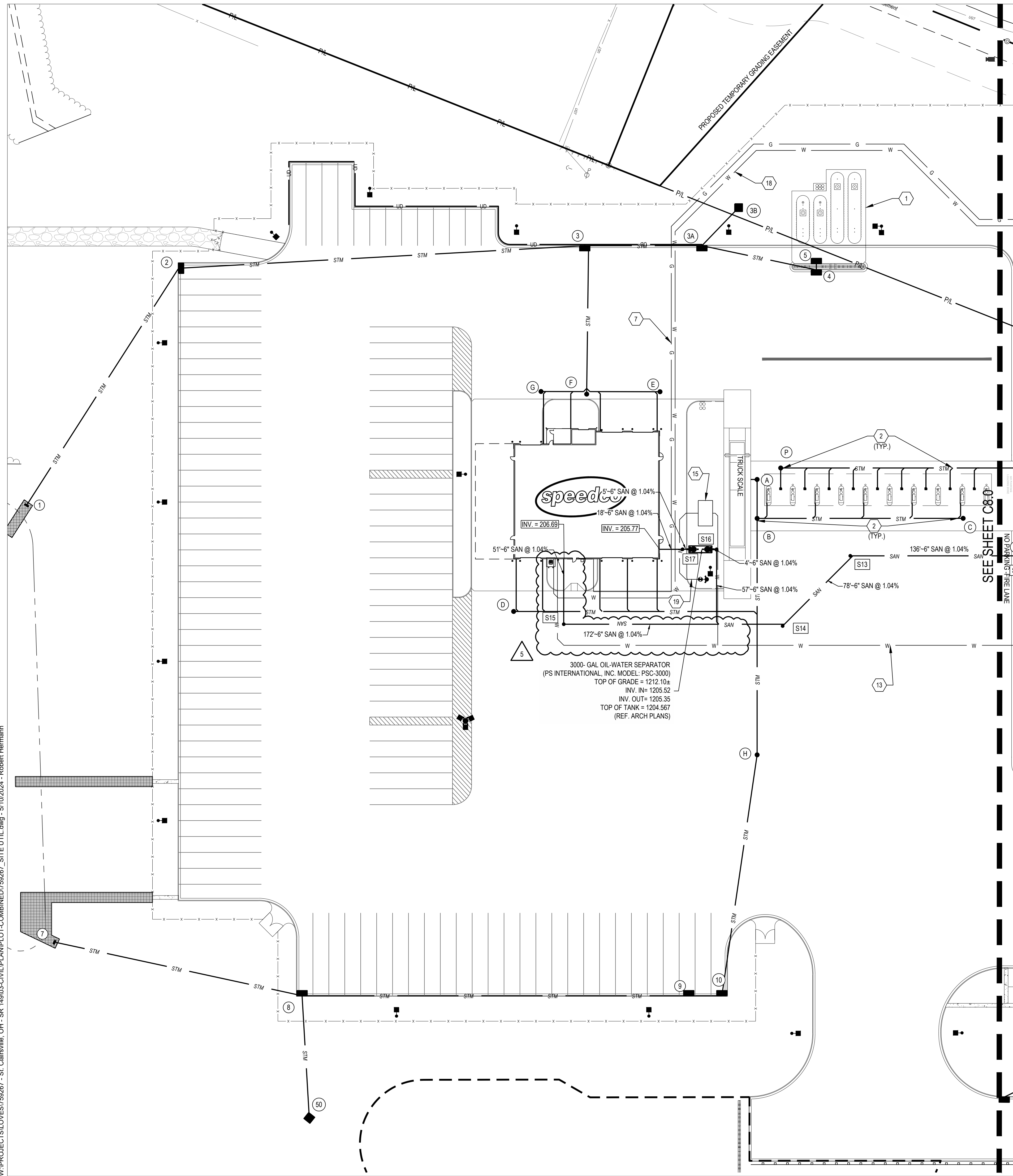
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FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



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LEGEND

- EXISTING**
REFER TO ALTAINSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- STM STORM SEWER
 - SAN SANITARY SEWER
 - W DOMESTIC WATER SERVICE
 - UGE UNDERGROUND ELECTRIC SERVICE
 - G GAS SERVICE
 - UGT TELEPHONE SERVICE
 - UTILITY EASEMENT
- (10) STORM STRUCTURE NUMBER
 (A) STORM CLEANOUT NUMBER
 (S) STORM STANDARD MANHOLE
 (C) STORM CURB INLET
 (E) STORM ENDWALL
 (D) YARD DRAIN
 (N) STORM CLEANOUT
 (S21) SANITARY STRUCTURE NUMBER
 (S) SANITARY CLEANOUT
 (V) WATER VALVE
 (M) WATER METER
 (R) REDUCER
 (N) BACKFLOW PREVENTER
 (F) FIRE HYDRANT
 (G) GAS METER
 (L) LIGHT POLE
 (C) TRUELOOK CAMERA

CODED NOTES:

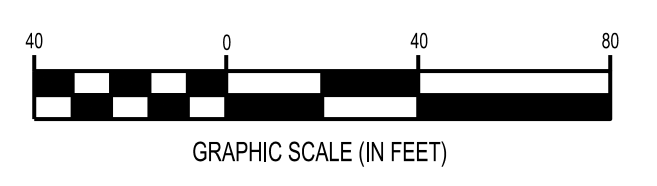
- UNDERGROUND STORAGE TANKS. REFERENCE ARCHITECTURAL PLANS AND FUELING PLANS.
- WYE CONNECTION.
- PROPANE STORAGE AREA. REFERENCE ARCHITECTURAL PLANS.
- AIR/ WATER ISLAND. REFERENCE ARCHITECTURAL PLANS.
- CONTRACTOR SHALL PROVIDE (2) - 4" CONDUITS TO BUILDING FOR TELEPHONE AND COMMUNICATIONS. COORDINATE WITH BUILDING CONTRACTOR AND TELEPHONE/COMMUNICATIONS COMPANY FOR CONNECTION AND EXACT LOCATION OF PROPOSED SERVICE.
- CONNECT TO EXISTING WATERMAIN. CONTRACTOR TO COORDINATE CONNECTION WITH BELMONT COUNTY WATER AND SEWER DISTRICT. REFER TO SHEET C9.4 FOR TIE-IN DETAILS.
- PROPOSED NATURAL GAS SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH GAS METER AND CONNECTION.
- PROPOSED UNDERGROUND ELECTRIC SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH ELECTRIC COMPANY.
- PROPOSED UNDERGROUND TELEPHONE SERVICE. CONTRACTOR TO COORDINATE FINAL LOCATION WITH TELEPHONE COMPANY.
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- PROPOSED 2" METER VAULT FOR BACK FLOW PREVENTER AND METER PER BELMONT COUNTY WATER AND SEWER DISTRICT DETAIL. REFER TO SHEET C9.4.
- PROPOSED FIRE PUMP HOUSE. REFER TO DETAIL SHEET C9.7 AND DESIGN BY FLUID SOLUTIONS.
- CORE DRILL EXISTING MANHOLE AND USE KOR-N-SEAL BOOT OR APPROVED EQUAL. CORE DRILL THRU OR MODIFY CHANNEL FOR FLOW.
- PROPOSED 8" PRIVATE WATER LINE.
- PROPOSED 1.25" PRIVATE WATER SERVICE LINE.
- PROPOSED 6" PRIVATE FIRE LINE SERVICE.
- PROPOSED 4" PRIVATE FIRE LINE SERVICE.
- PROPOSED 3" PRIVATE WATER SERVICE LINE.

GENERAL NOTES:

- AS CONSTRUCTION ACTIVITIES APPROACH AN EXISTING UTILITY LINE, THE CONTRACTOR SHALL LOCATE THE LINE, DETERMINE ITS EXACT LOCATION, AND MAKE GRADE ADJUSTMENTS BY DEFLECTING THE PIPELINE FITTINGS. MAXIMUM FITTING DEFLECTION SHALL BE PER MANUFACTURER'S WRITTEN INSTRUCTIONS.
- CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATION OF VESTIBULE, CANOPY, SIDEWALKS, EXIT PORCHES, RAMPS, DRIVE THRU, AND EXACT BUILDING UTILITY ENTRANCE LOCATIONS.
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- PROVIDE THRUST BLOCKS AT VALVES, TEES, FITTINGS AND ALL HORIZONTAL AND VERTICAL CHANGES IN DIRECTION OF THE WATER LINE. PERFORM ALL WATER LINE WORK IN ACCORDANCE WITH BELMONT COUNTY WATER AND SEWER DISTRICT STANDARDS.
- REFER TO PLUMBING SITE PLAN IN ARCHITECTURAL SET FOR ADDITIONAL UNDERGROUND UTILITIES.
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- LINE UNDERGROUND SHALL BE INSTALLED, INSPECTED AND APPROVED PRIOR TO BACKFILLING.
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- COORDINATE UTILITIES WITH PLUMBING CONTRACTOR. CAP AND MARK FOR FUTURE CONNECTION. FINAL CONNECTION BY PLUMBING CONTRACTOR.
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- REFER TO STORM SEWER PROFILES SHEETS C7.13 & C7.15 FOR LENGTH AND SLOPE OF STORM PIPE.
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- TRUELOOK SITE CAMERA AND COMPONENTS SHALL BE PROVIDED BY OWNER. CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION, MAINTENANCE, AND REMOVAL AS PER THE TRUELOOK CAMERA INSTALLATION GUIDE. CAMERA TO BE POLE MOUNTED APPROXIMATELY 35' IN THE AIR WITH 6' BURIED, AND SHALL BE PLACED AS SHOWN ON THE PLANS OR IN A LOCATION THAT OFFERS MAXIMUM SITE VISIBILITY, AND WHICH WILL NOT BE DISTURBED OR MOVED DURING CONSTRUCTION.

REFER TO SHEET C8.0 FOR UTILITY GENERAL NOTES

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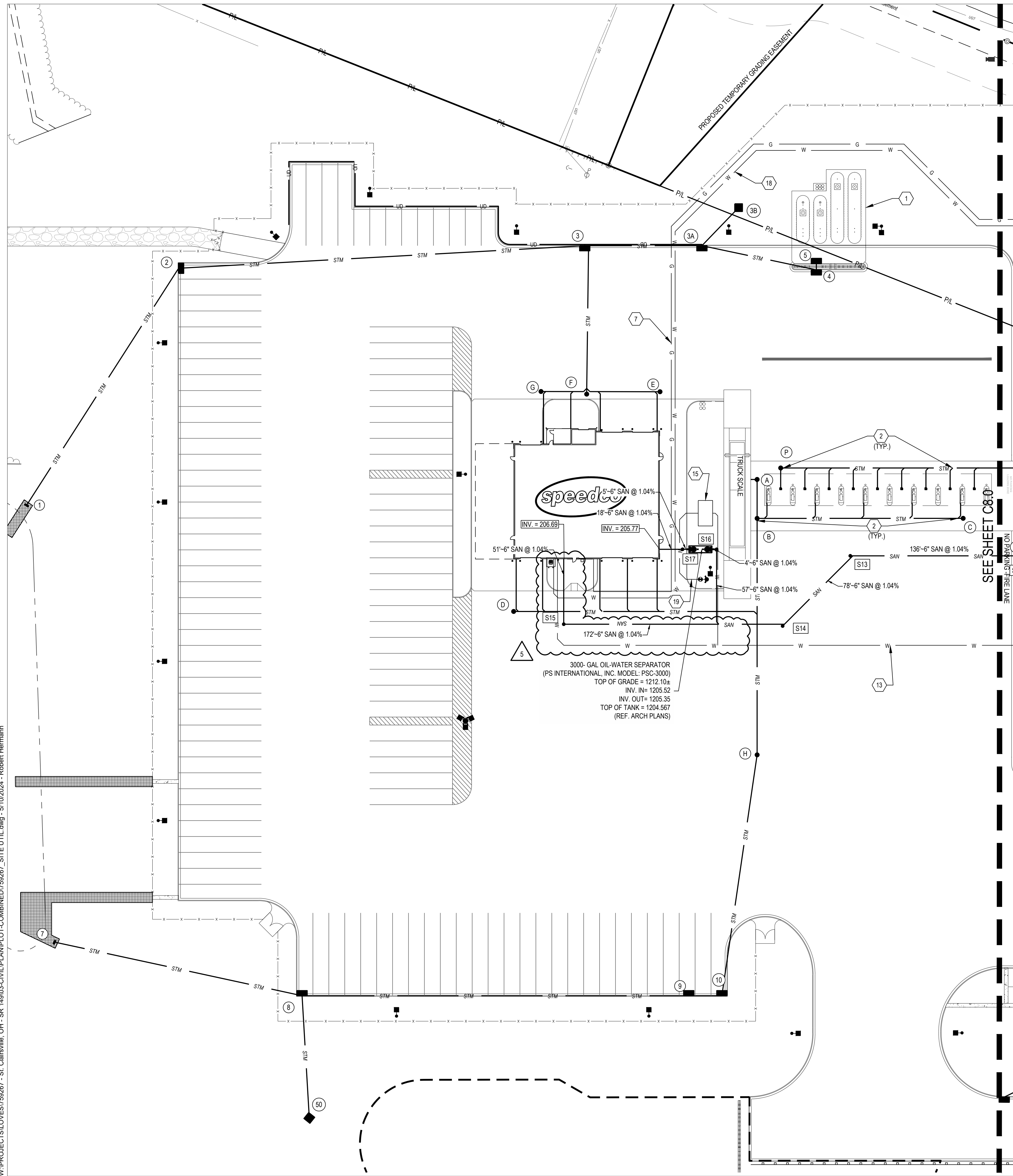
Revisions / Submissions		
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1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024
5	ADDENDUM 5	04/18/2024

Project Number:	759267
Scale:	1" = 40'
Drawn By:	FAR
Checked By:	JTK
Date:	12/19/2023
Issue:	OUT TO BID

Drawing Title:
UTILITY PLAN - WEST

C8.1

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE UTIL.dwg - 5/10/2024 - Robert Hermann



LEGEND

- EXISTING**
REFER TO ALTAINSPS TOPOGRAPHIC SURVEY
- PROPOSED**
- STM STORM SEWER
 - SAN SANITARY SEWER
 - W DOMESTIC WATER SERVICE
 - UGE UNDERGROUND ELECTRIC SERVICE
 - G GAS SERVICE
 - UGT TELEPHONE SERVICE
 - UTILITY EASEMENT
- (10) STORM STRUCTURE NUMBER
 (A) STORM CLEANOUT NUMBER
 (S) STORM STANDARD MANHOLE
 (C) STORM CURB INLET
 (E) STORM ENDWALL
 (D) YARD DRAIN
 (●) STORM CLEANOUT
 (S21) SANITARY STRUCTURE NUMBER
 (●) SANITARY CLEANOUT
 (V) WATER VALVE
 (M) WATER METER
 (R) REDUCER
 (N) BACKFLOW PREVENTER
 (H) FIRE HYDRANT
 (G) GAS METER
 (L) LIGHT POLE
 (C) TRUELOOK CAMERA

CODED NOTES:

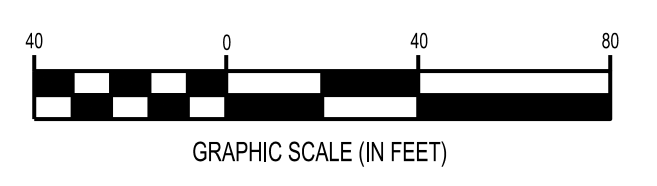
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NO. C12

STF CLEANO

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ST. CLAIRSVILLE, OH

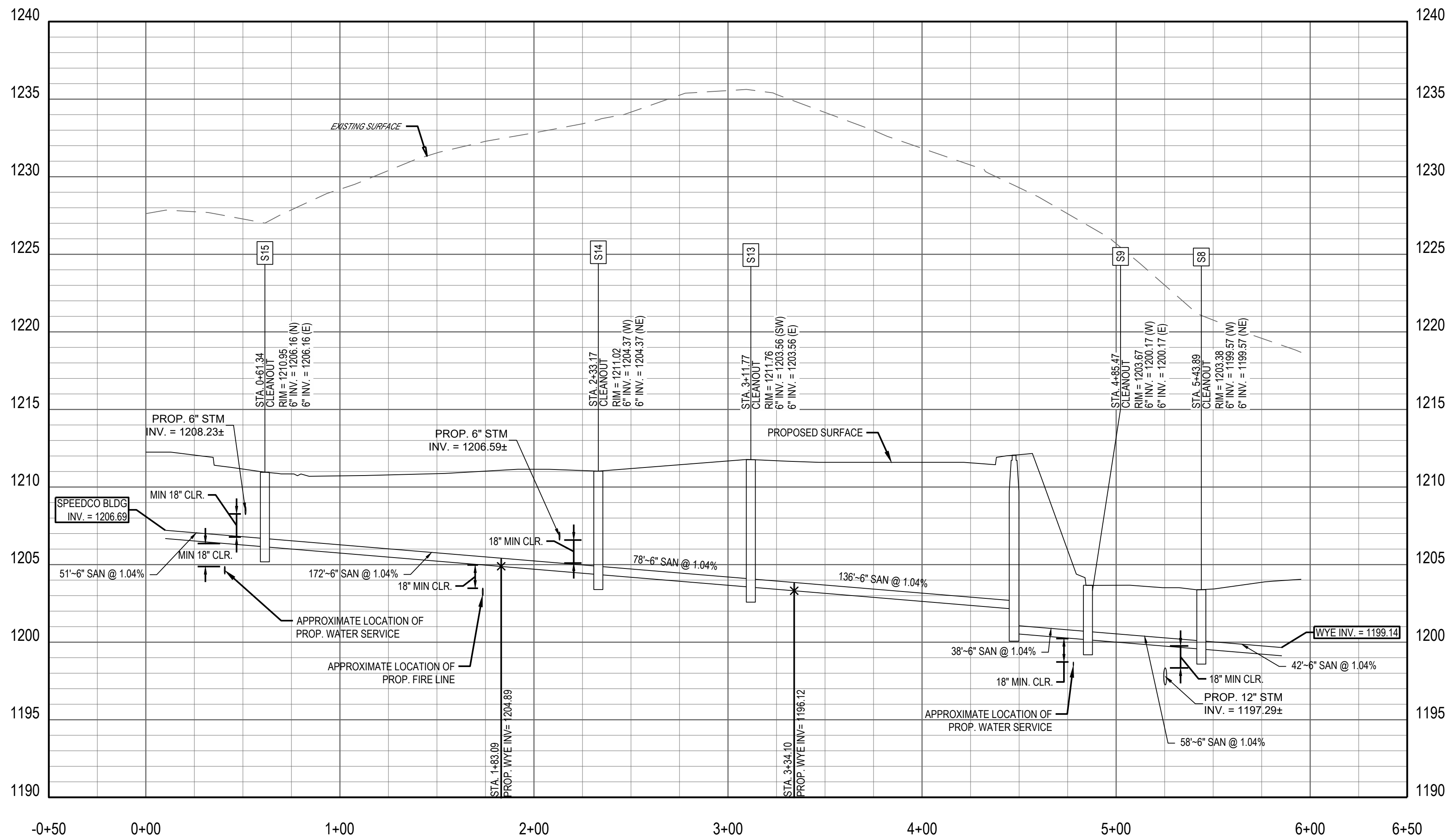
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions		
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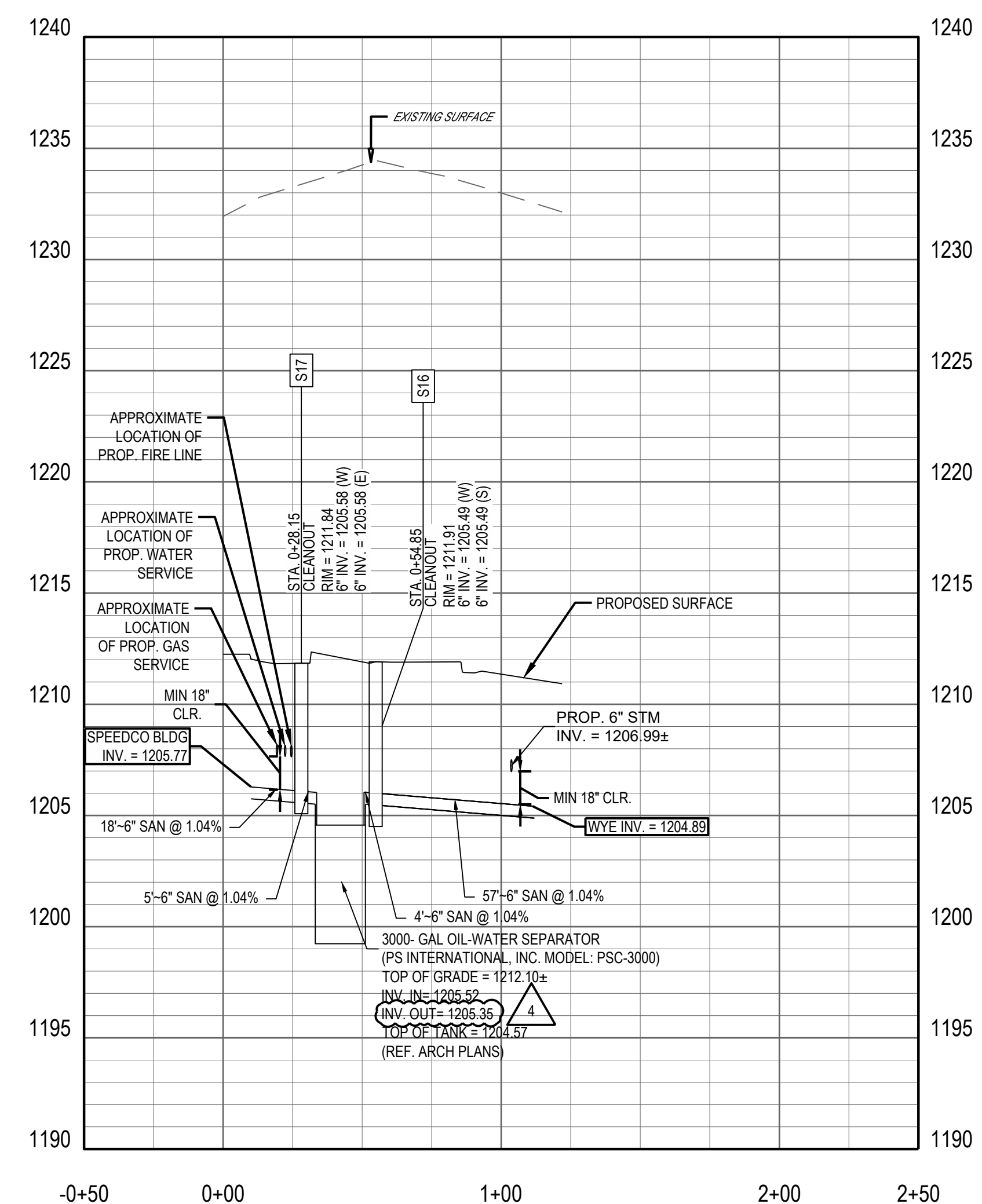
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Drawn By:	FAR
Checked By:	JTK
Date:	12/19/2023
Issue:	OUT TO BID

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UTILITY PLAN - WEST

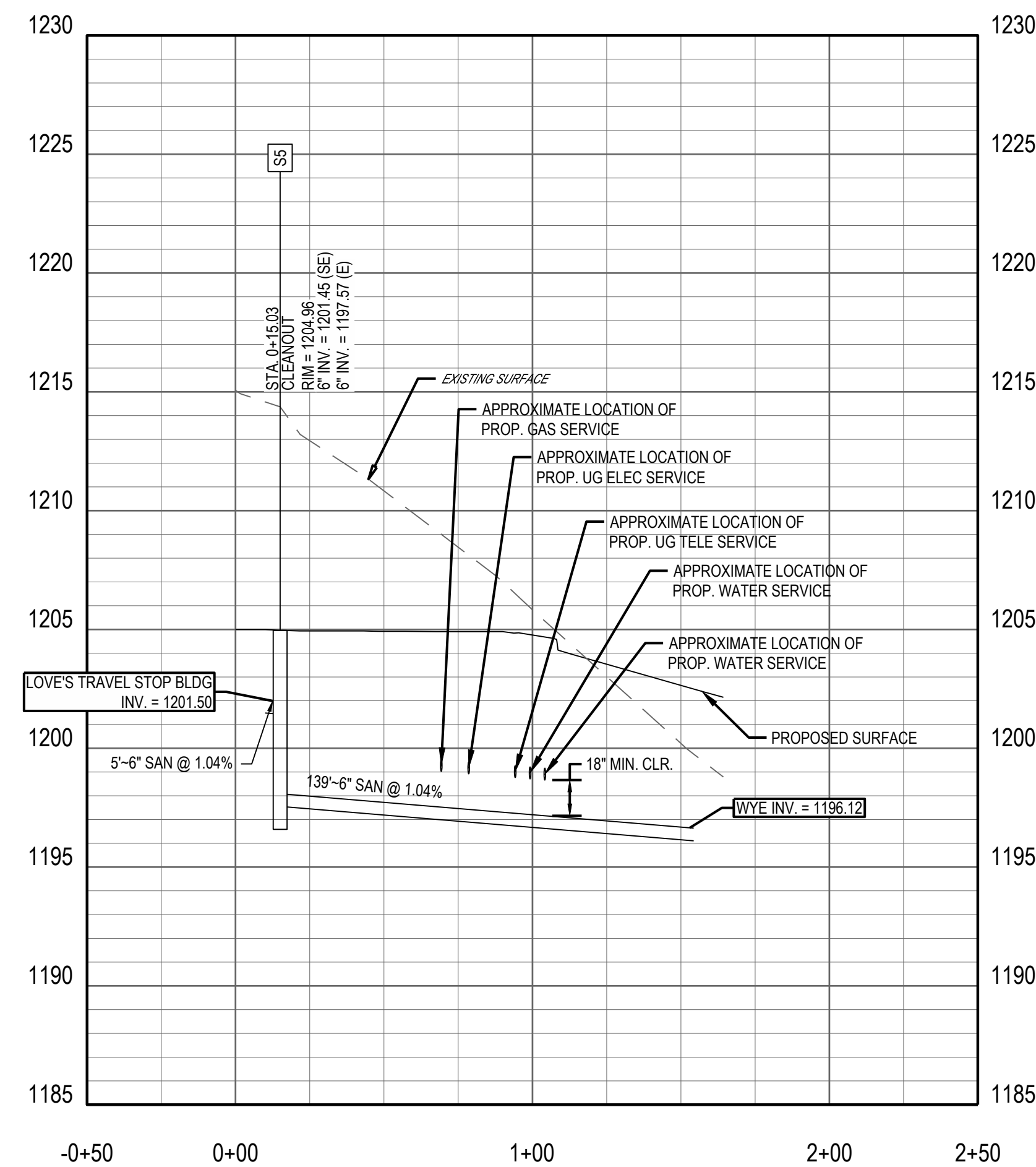
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SANITARY STRC 15-8 PROFILE
 SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.



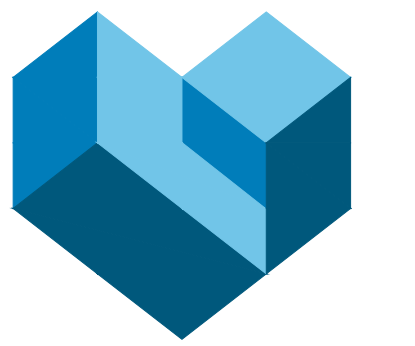
SANITARY STRC 17-16 PROFILE
 SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.



SANITARY STRC S5 PROFILE
 SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.

LEGEND
 - - - - - EXISTING GRADE
 ———— PROPOSED GRADE

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LOVES TRAVEL STOP

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 66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

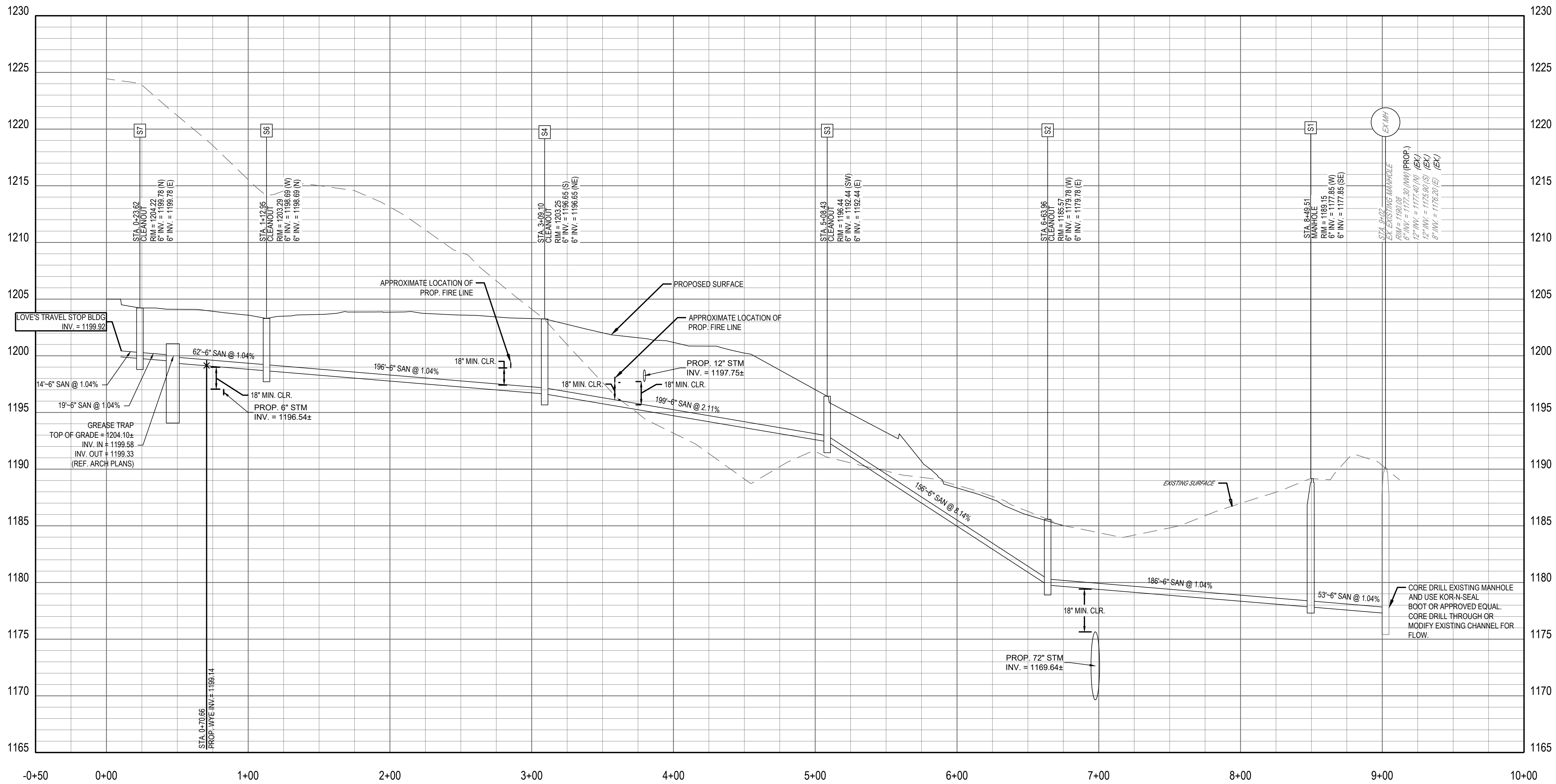
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ID	Description	Date
1	REV 1	01/03/2024
4	ADDENDUM 4	02/23/2024

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 Checked By: JTK
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Drawing Title:
SANITARY PROFILE

C8.2

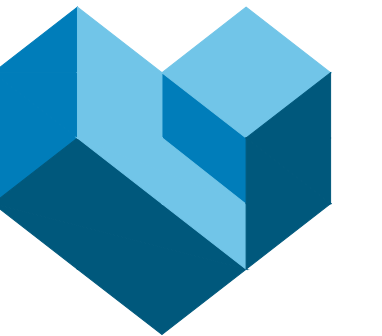
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SANITARY STRC 7 - EX. MH PROFILE
SCALE: 1" = 40' HORIZ.; 1" = 5' VERT.

LEGEND
 --- EXISTING GRADE
 ——— PROPOSED GRADE

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LOVE'S TRAVEL STOP

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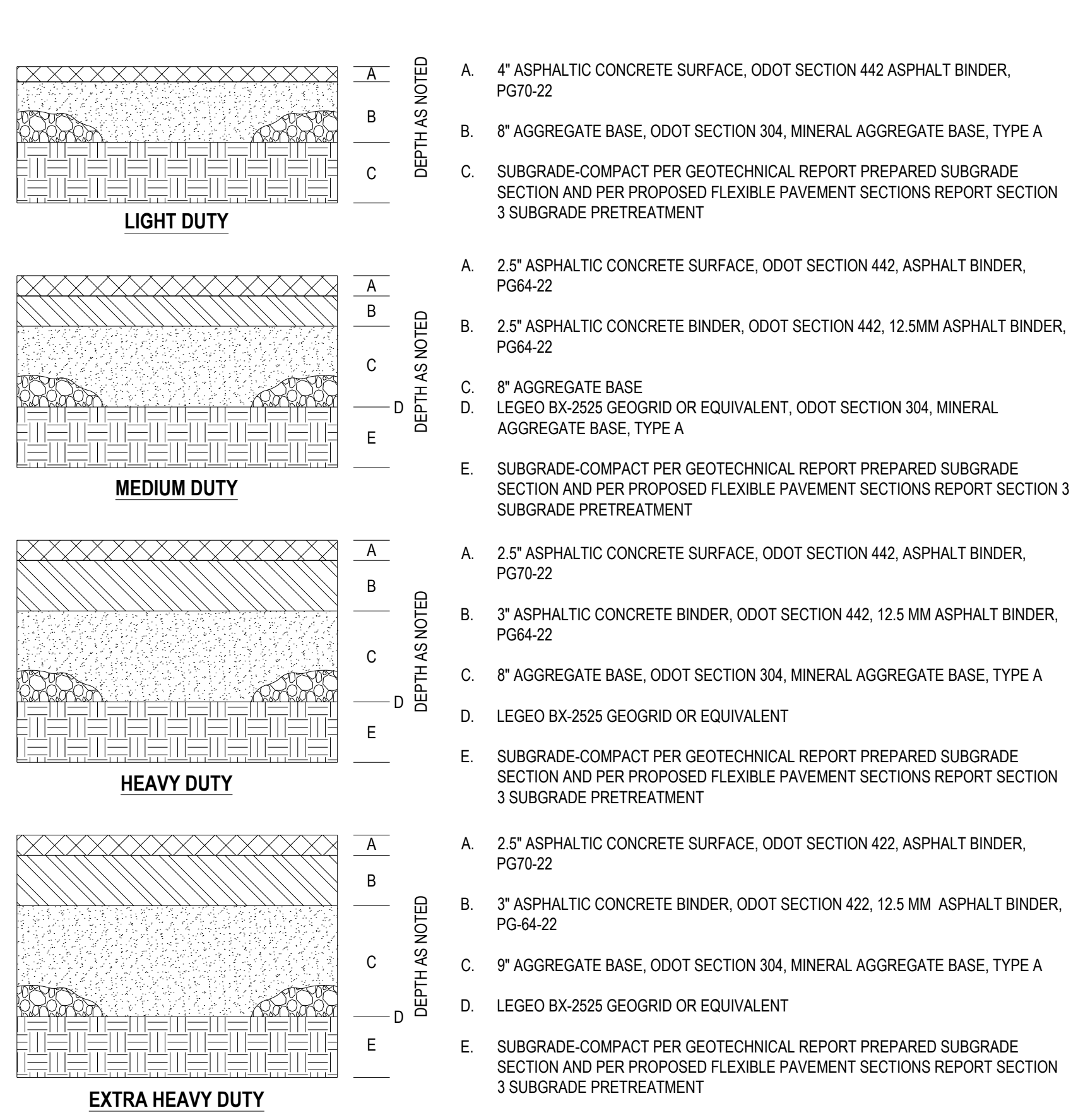
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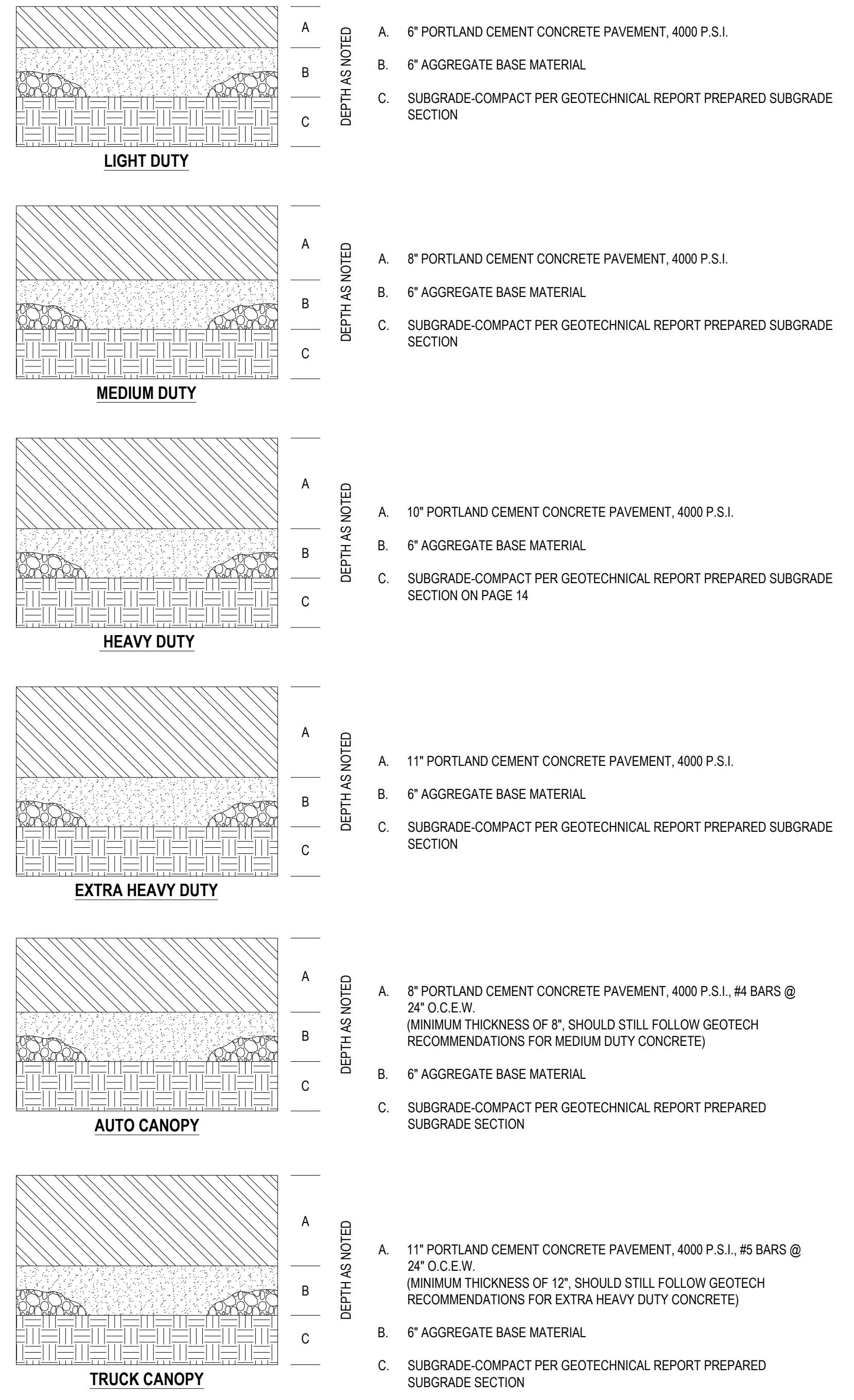
Drawing Title:
SANITARY PROFILE

C8.3

ASPHALTIC PAVEMENT



CONCRETE PAVEMENT



ON-SITE PAVEMENT NOTES

- ALL CONSTRUCTION METHODS AND MATERIALS SHALL BE IN ACCORDANCE WITH THE ODOT STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MAINTENANCE OF HIGHWAYS, STREETS, AND BRIDGES.
- PAVEMENT DESIGN IS BASED ON A 20 YEAR DESIGN LIFE. (REFER TO GEOTECHNICAL ENGINEERING REPORT.)
- CONCRETE SHALL BE AIR ENTRAINED AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 P.S.I. AFTER 28 DAYS OF LABORATORY CURING. REFER TO GEOTECHNICAL REPORT FOR CONCRETE JOINT RECOMMENDATIONS.
- REFER TO GEOTECHNICAL REPORT FOR SUBGRADE PREPARATION.
- ALL PROPOSED AGGREGATE BASE FOR CONCRETE PAVEMENT SHALL CONFORM TO ODOT MATERIAL SPECIFICATION SECTION 815 PER THE GEOTECHNICAL REPORT.

GENERAL JOINTING NOTES:

- NOTES AND DETAILS BASED ON ACI 330.2R-17 AND 330.8R GUIDELINES. ALL RECOMMENDATIONS SHOULD BE CONFIRMED ON A PROJECT BY PROJECT BASIS WITH THE CIVIL AND/OR GEOTECHNICAL ENGINEER. IF THE DETAILS AND RECOMMENDATIONS PROVIDED DO NOT COMPLY WITH THE STANDARDS OF THE LATEST INDUSTRY STANDARDS, OR THE STANDARDS ESTABLISHED BY THE AUTHORITY HAVING JURISDICTION, THIS SHOULD BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE OWNER'S REPRESENTATIVE.
- CONSTRUCTION AND TRANSVERSE JOINTS SHALL BE RE-SAWN AND ALL SAW LAITANCE VACUUMED FROM THE JOINT ONCE BOTH SIDES ARE POURED.
- PAVEMENT JOINTS OF ANY TYPE SHOULD EXTEND THROUGH ANY CURB AND GUTTER.
- CONCRETE CONTRACTOR SHALL SUBMIT THE CONCRETE MIX TO THE OWNER'S REPRESENTATIVE FOR REVIEW AND APPROVAL.
- A SCALED DRAWING OF THE JOINTING PLAN FOR THE PAVEMENTS AND CURBS SHOULD BE SUBMITTED BY THE CONTRACTOR FOR REVIEW BY THE CIVIL AND GEOTECHNICAL ENGINEERS.

SIDEWALK JOINT NOTES

- ALL JOINTS TO BE SEALED TO BE THOROUGHLY CLEANED BY HYDROBLASTING AND/OR SAND BLASTING METHODS. THE JOINTS ARE TO BE FREE FROM ALL DUST COATINGS, ANY CONTAMINATES, AND FREE FROM ALL MOISTURE THAT MIGHT INTERFERE WITH THE PROPER AND SATISFACTORY BONDING OF THE JOINT SEALANT MATERIAL. THE JOINT WILL BE BLOWN OUT WITH DRY COMPRESSED AIR IMMEDIATELY PRIOR TO APPLYING SEALANT.
- CONSTRUCTION EQUIPMENT AND OTHER VEHICLES AND PEDESTRIANS WHICH MAY CAUSE DAMAGE TO THE JOINTS SHALL NOT BE ALLOWED ON THE PAVEMENT AND SIDEWALKS BEFORE THE JOINT SEALANT BECOMES TACK FREE.
- USE 1/2" x 4" EXPANSION JOINT MATERIAL ALONG SIDEWALK CURB AND ALONG BUILDING.
- USE 1/2" x 4" EXPANSION JOINT MATERIAL AROUND POLES OR OTHER OBSTRUCTIONS IN WALK.
- SAW CUT JOINTS WITHIN 24 HOURS.

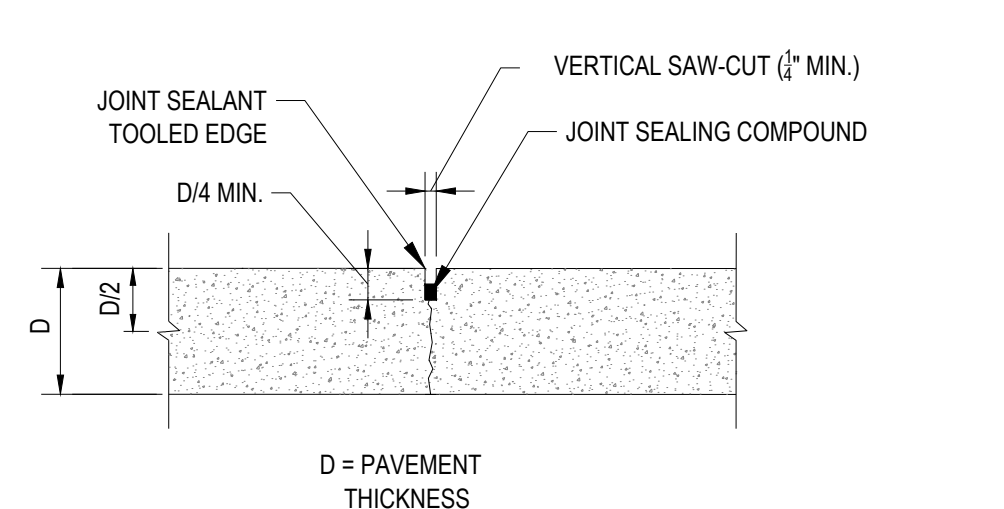


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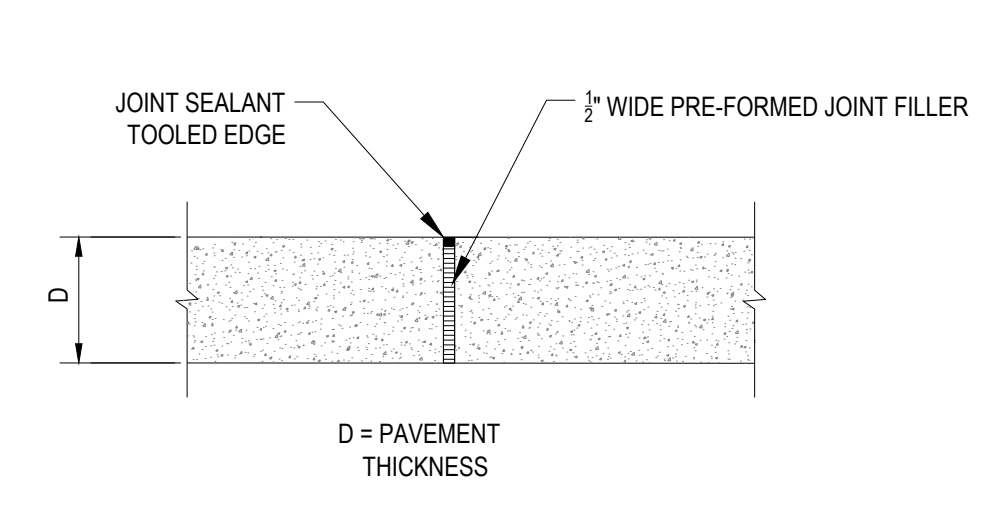
- NOTES:
- DOWELS SHALL BE SUPPORTED BY AN APPROVED DEVICE.
 - ALL BARS AND BASKETS SHALL BE EPOXY COATED.
 - DOWELED CONTRACTION JOINTS ARE REQUIRED WHERE PAVEMENT DEPTH IS 7" OR GREATER.
 - DOWELS SHALL BE SMOOTH BARS AND FREE MOVEMENT SHALL BE PROVIDED BY APPLYING A COATING OF GREASE AS A BOND-BREAKING MATERIAL JUST PRIOR TO PLACING THE CONCRETE.

DOWELED CONTRACTION JOINT DETAIL

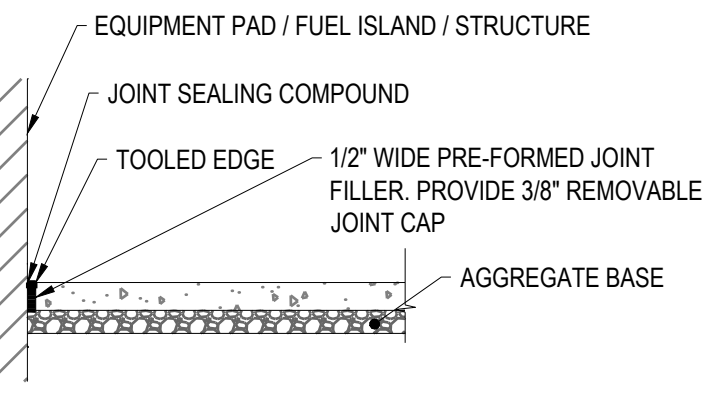


- NOTES:
- MAXIMUM CONTRACTION JOINT SPACING IS DEPENDENT UPON THE PAVEMENT THICKNESS. REFER TO TABLE BELOW FOR SPACING REQUIREMENTS.
 - ENSURE CONTRACTION JOINT PATTERNS DIVIDE PAVEMENT INTO PANELS AS CLOSE TO SQUARE AS POSSIBLE.

CONTRACTION JOINT DETAIL

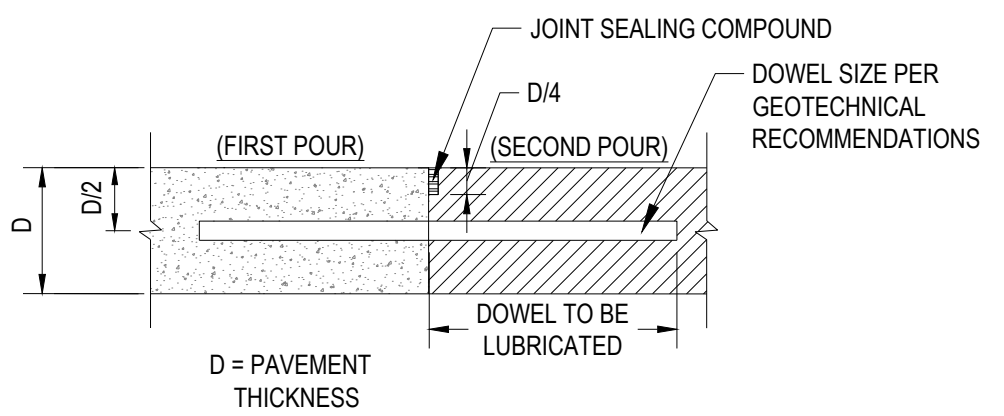


PAVEMENT THICKNESS (IN)	MAXIMUM SPACING (FT)
4 - 4.5	10
5 - 5.5	12.50
6 OR GREATER	15



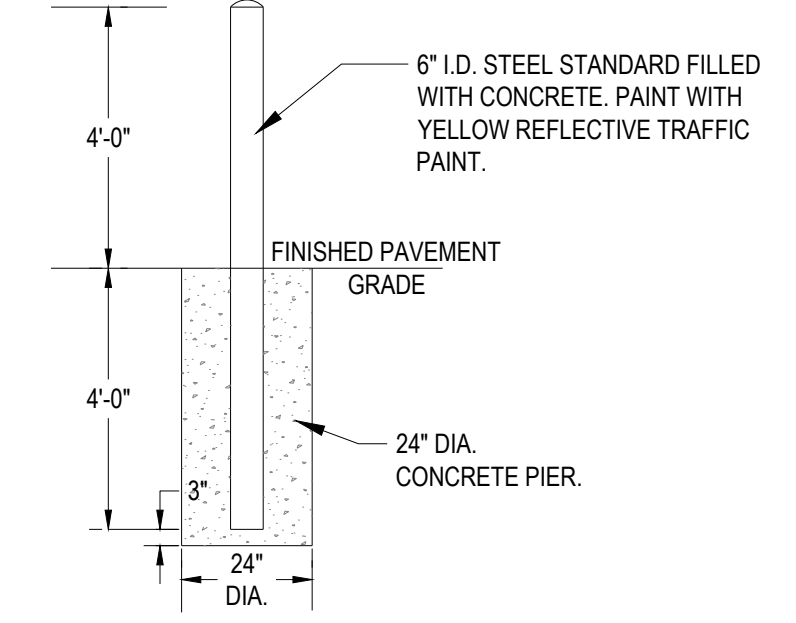
- NOTES:
- PLACE ISOLATION JOINTS WHERE CONCRETE PAVEMENT ABUTS SLABS, FUEL ISLANDS, CANOPY COLUMNS, BOLLARDS AND STRUCTURES OR FIXED OBJECTS.
 - PRE-FORMED JOINT FILLER - NON-IMPREGNATED TYPE, CLOSED CELL RESILIENT POLYETHYLENE FOAM, 1/2" THICK UNLESS OTHERWISE NOTED.
 - WHERE SLABS OF DIFFERENT THICKNESSES COME TOGETHER AT ISOLATION JOINTS, THE SUBGRADE/SUBBASE UNDER THE THINNER PAVEMENT SECTIONS SHOULD BE SHAPED TO PROVIDE GRADUAL THICKNESS TRANSITION OVER A DISTANCE OF 4 FT OR MORE.

ISOLATION JOINT DETAIL

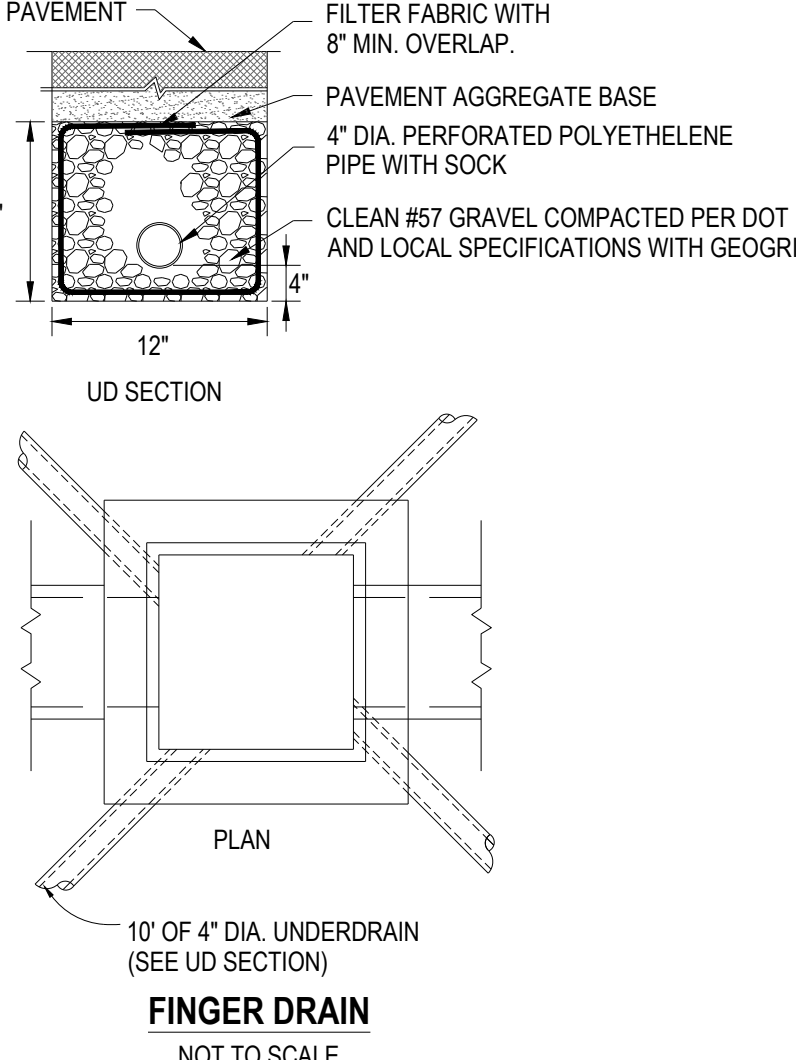


- NOTES:
- DOWELS SHALL BE SUPPORTED BY AN APPROVED DEVICE.
 - ALL BARS AND BASKETS SHALL BE EPOXY COATED.
 - PLACE CONSTRUCTION JOINTS AT END OF PLACEMENTS AND AT LOCATIONS WHERE PLACEMENT OPERATIONS ARE STOPPED FOR A PERIOD OF MORE THAN 1/2 HOUR.
 - DOWELS SHALL BE SMOOTH BARS AND FREE MOVEMENT SHALL BE PROVIDED BY APPLYING A COATING OF GREASE AS A BOND-BREAKING MATERIAL JUST PRIOR TO PLACING THE CONCRETE.
 - IN EMERGENCY SITUATIONS SUCH AS LACK OF MATERIALS, SUDDEN CHANGES IN WEATHER, OR EQUIPMENT BREAKDOWN, A CONSTRUCTION JOINT SHOULD BE INSTALLED IN PLACE OF THE NEAREST CONTRACTION JOINT LOCATION.
 - CONSTRUCTION JOINTS ARE NOT TO BE LOCATED CLOSER THAN 10'-0" TO ANOTHER PARALLEL JOINT.
 - WHERE SLABS OF DIFFERENT THICKNESSES COME TOGETHER AT JOINTS, THE SUBGRADE/SUBBASE UNDER THE THINNER PAVEMENT SECTIONS SHOULD BE SHAPED TO PROVIDE GRADUAL THICKNESS TRANSITION OVER A DISTANCE OF 4 FT OR MORE.

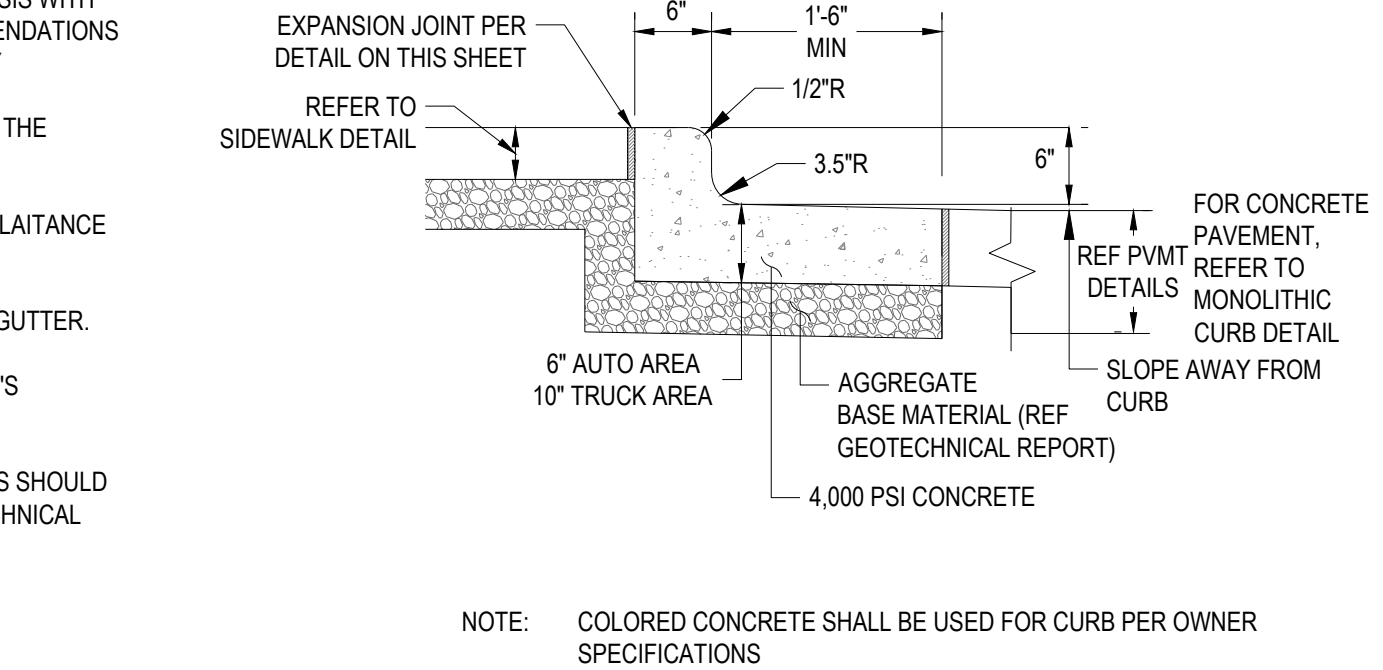
CONSTRUCTION JOINT DETAIL



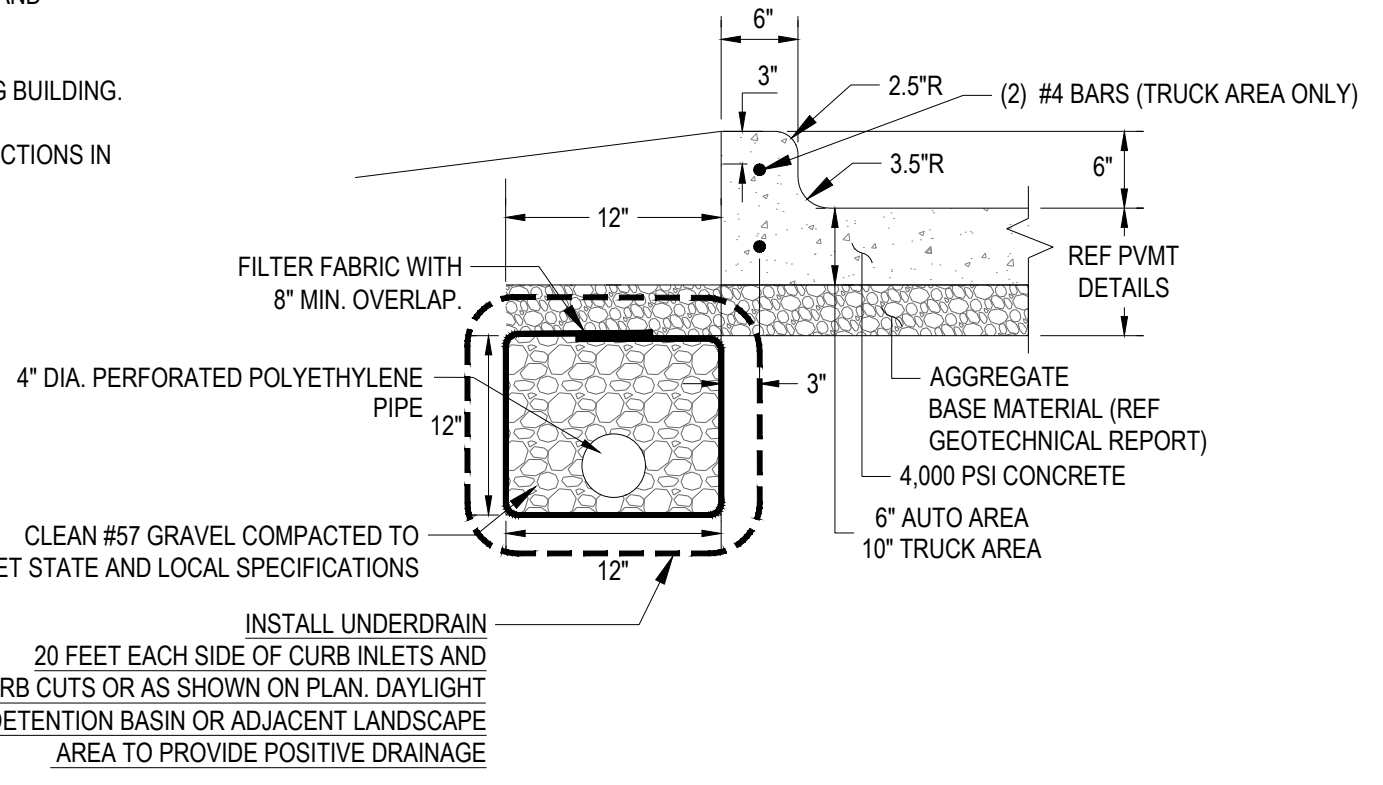
BOLLARD DETAIL



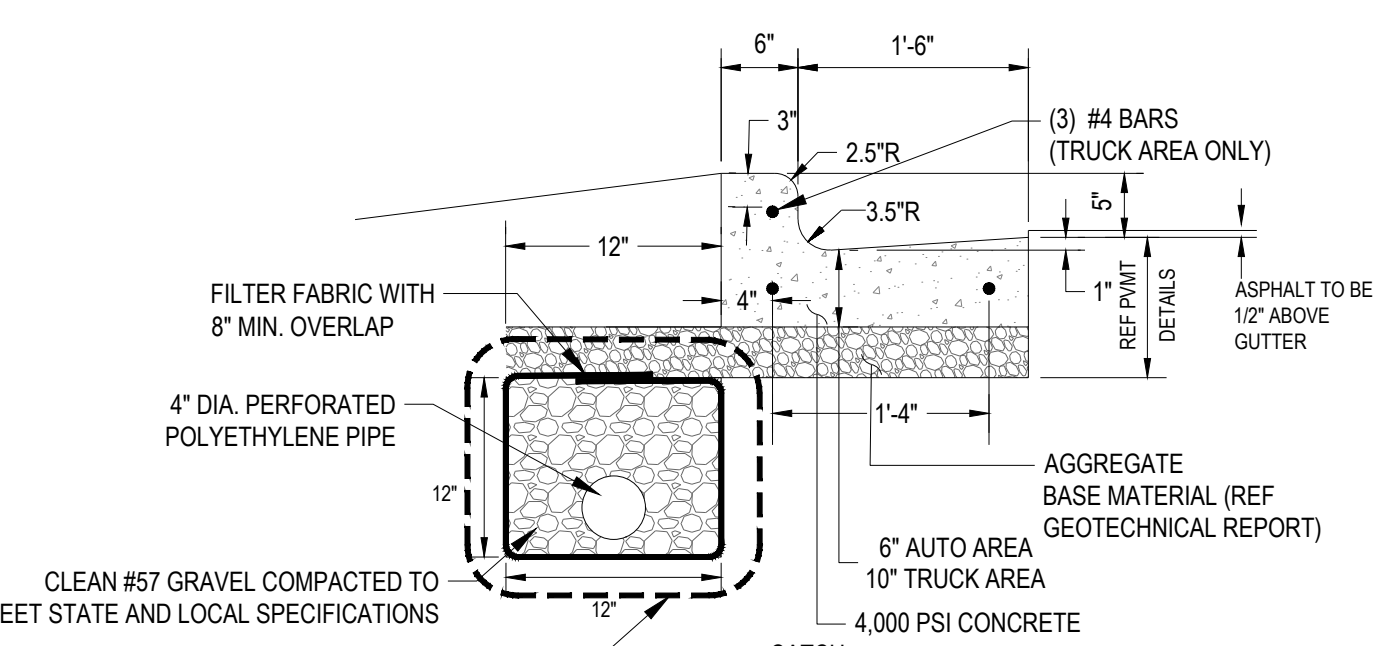
FINGER DRAIN



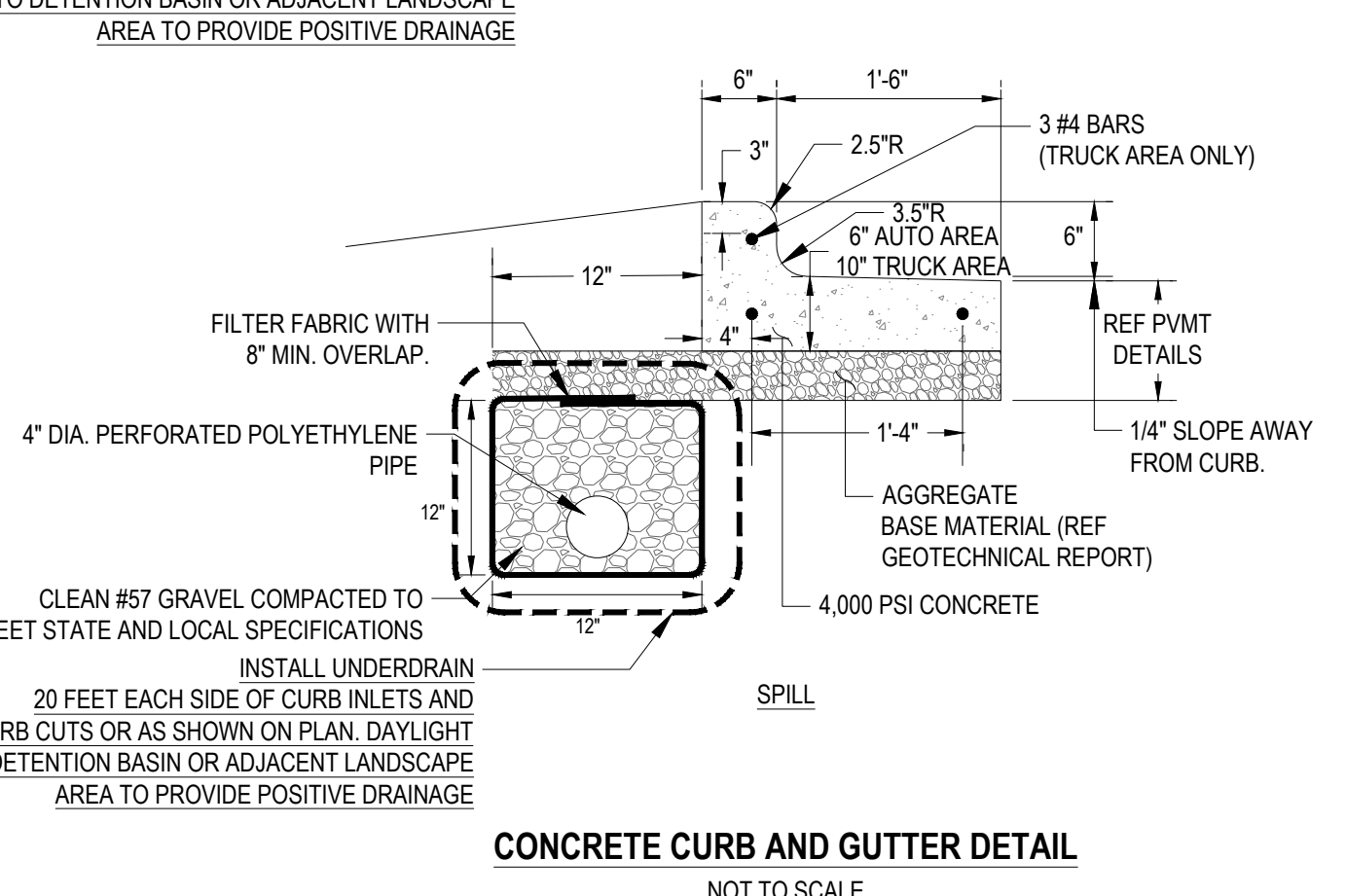
COLOR CONCRETE CURB DETAIL



6" MONOLITHIC CONCRETE CURB DETAIL



CONCRETE CURB AND GUTTER DETAIL



SPILL

LOVE'S TRAVEL STOP

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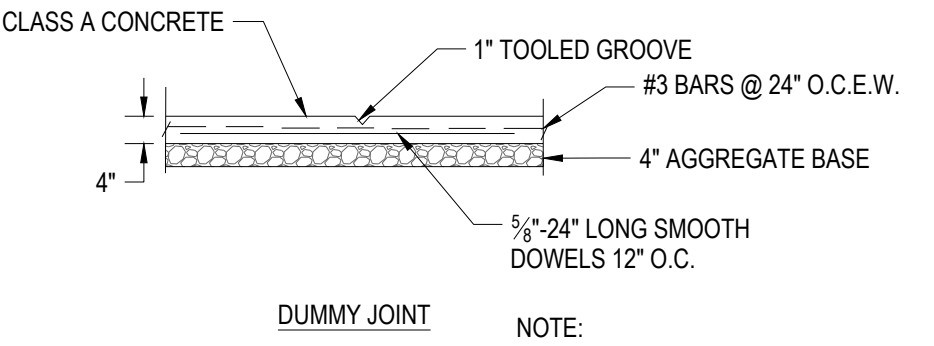
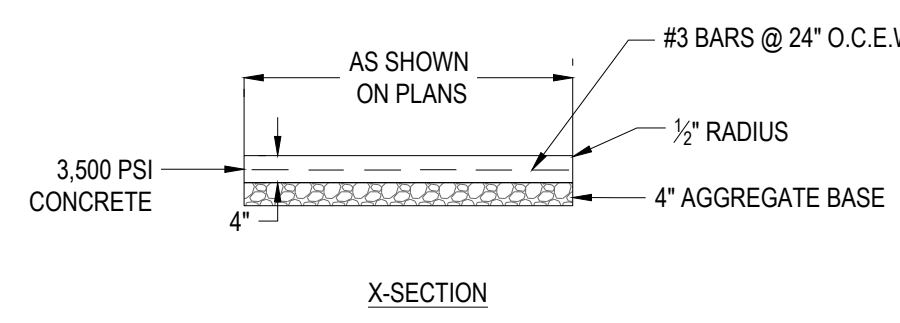
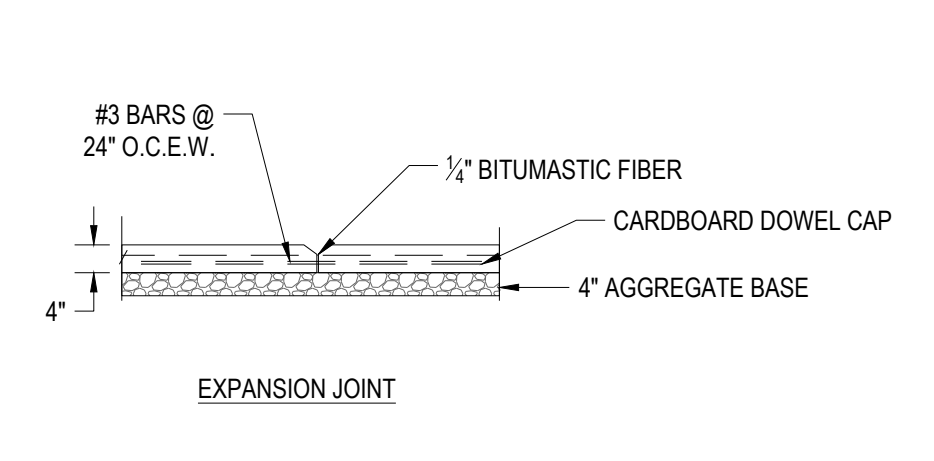
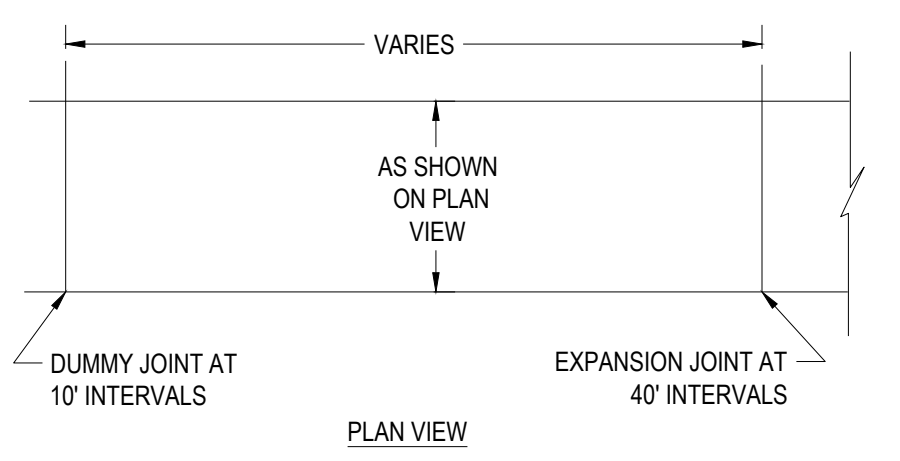
Revisions / Submissions

ID	Description	Date

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Project Number: 759267
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Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

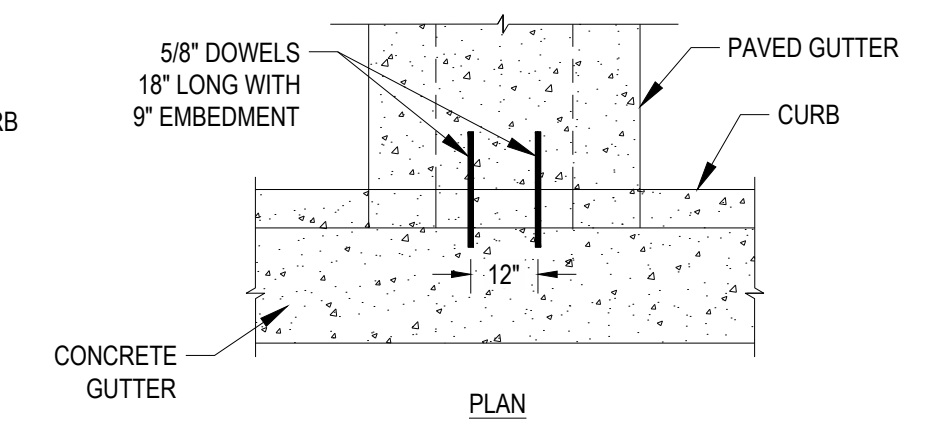
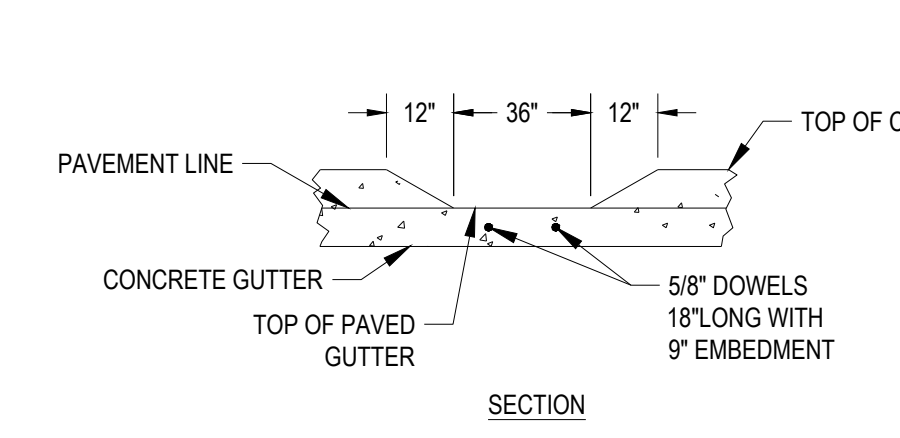
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CONSTRUCTION DETAILS
C9.0

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR. 149\03-CIVIL\PLAN\DOT-COMBINED\759267 SITE CONSTRUCTION DETAILS.dwg - 12/18/2023 - Francesca Rapposelli



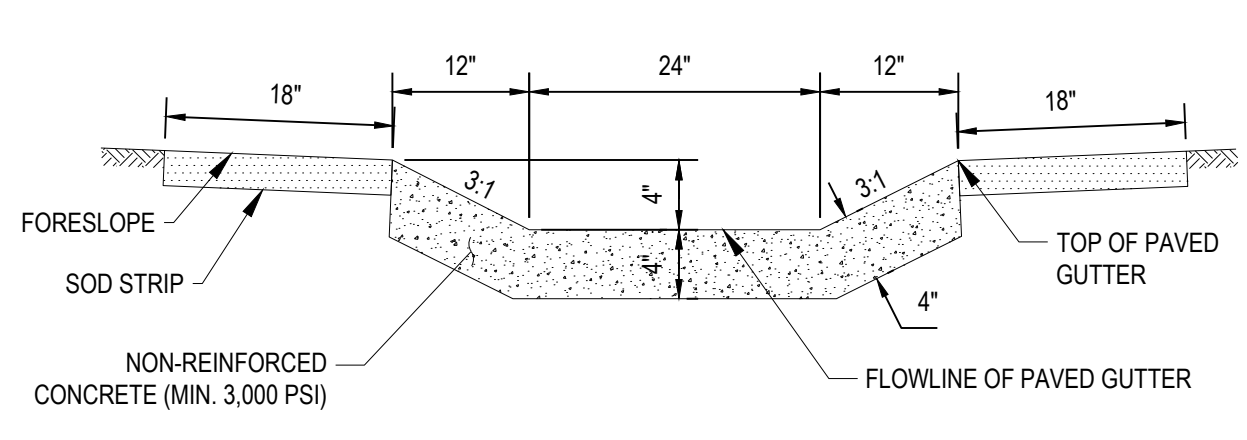
SIDEWALK DETAIL
NOT TO SCALE

NOTE: REFERENCE GEOTECHNICAL REPORT FOR JOINT AND DOWEL SPECIFICATIONS



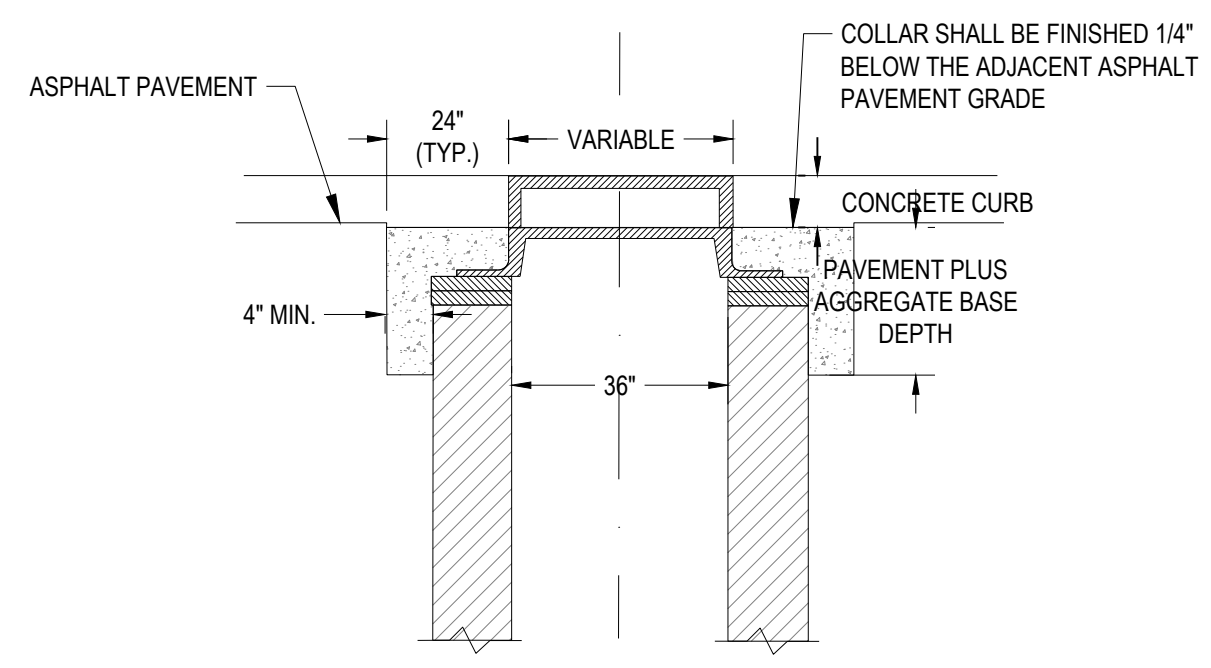
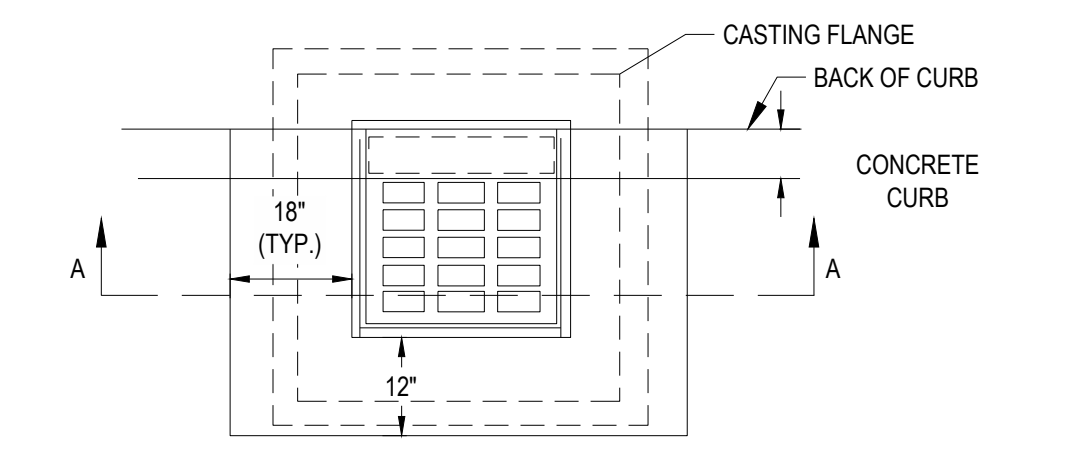
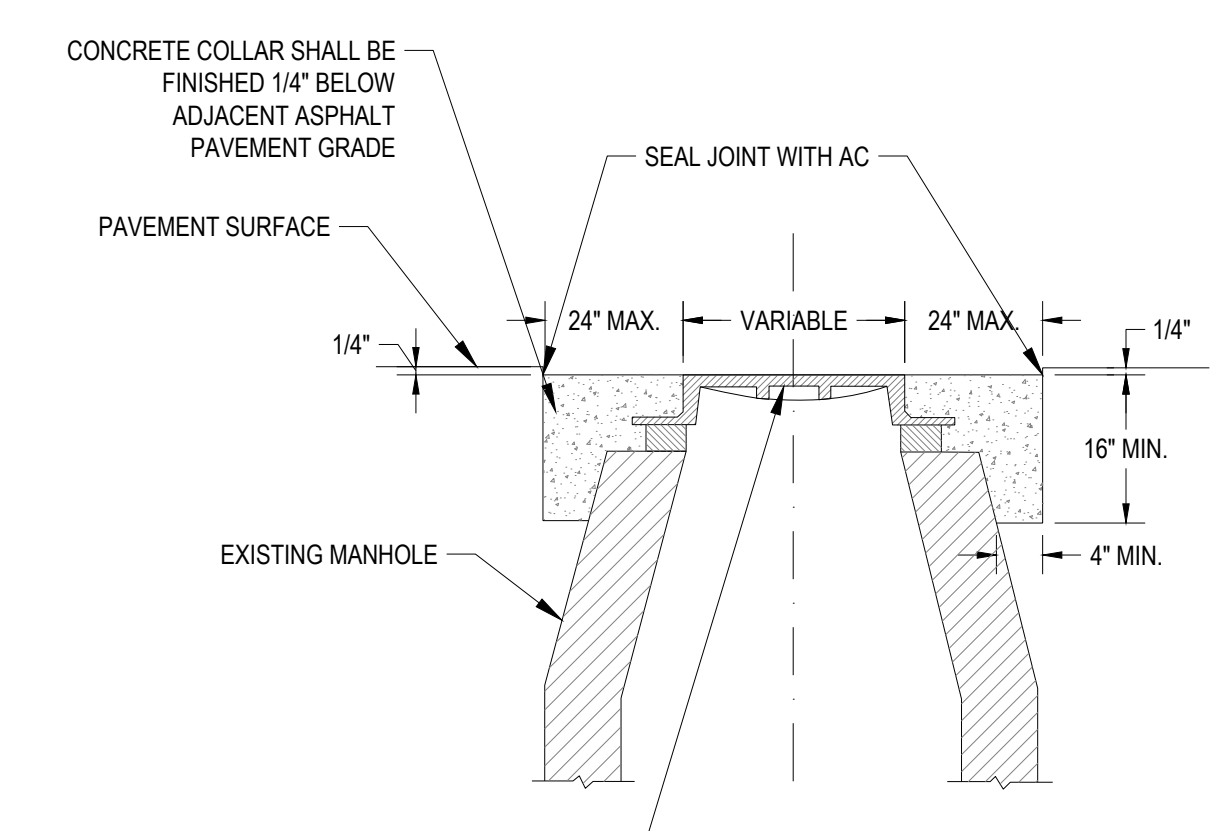
NOTE: CONTRACTOR TO MATCH TOP OF PAVED GUTTER TO TOP OF CURB AND THEN TRANSITION GUTTER DEPTH DOWN TO 4" OVER 6'.

CURB CUT DETAIL
NOT TO SCALE



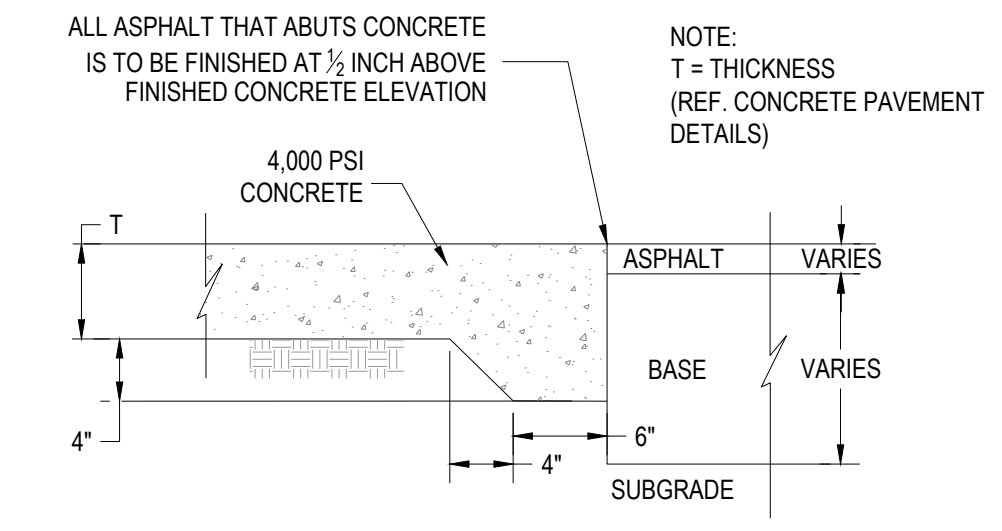
NOTE: FLOWLINE OF PAVED GUTTER TO MATCH CURB CUT WIDTH OF 36" AT OPENING, THEN TRANSITION TO 24" WITHIN 6'

PAVED GUTTER
NOT TO SCALE

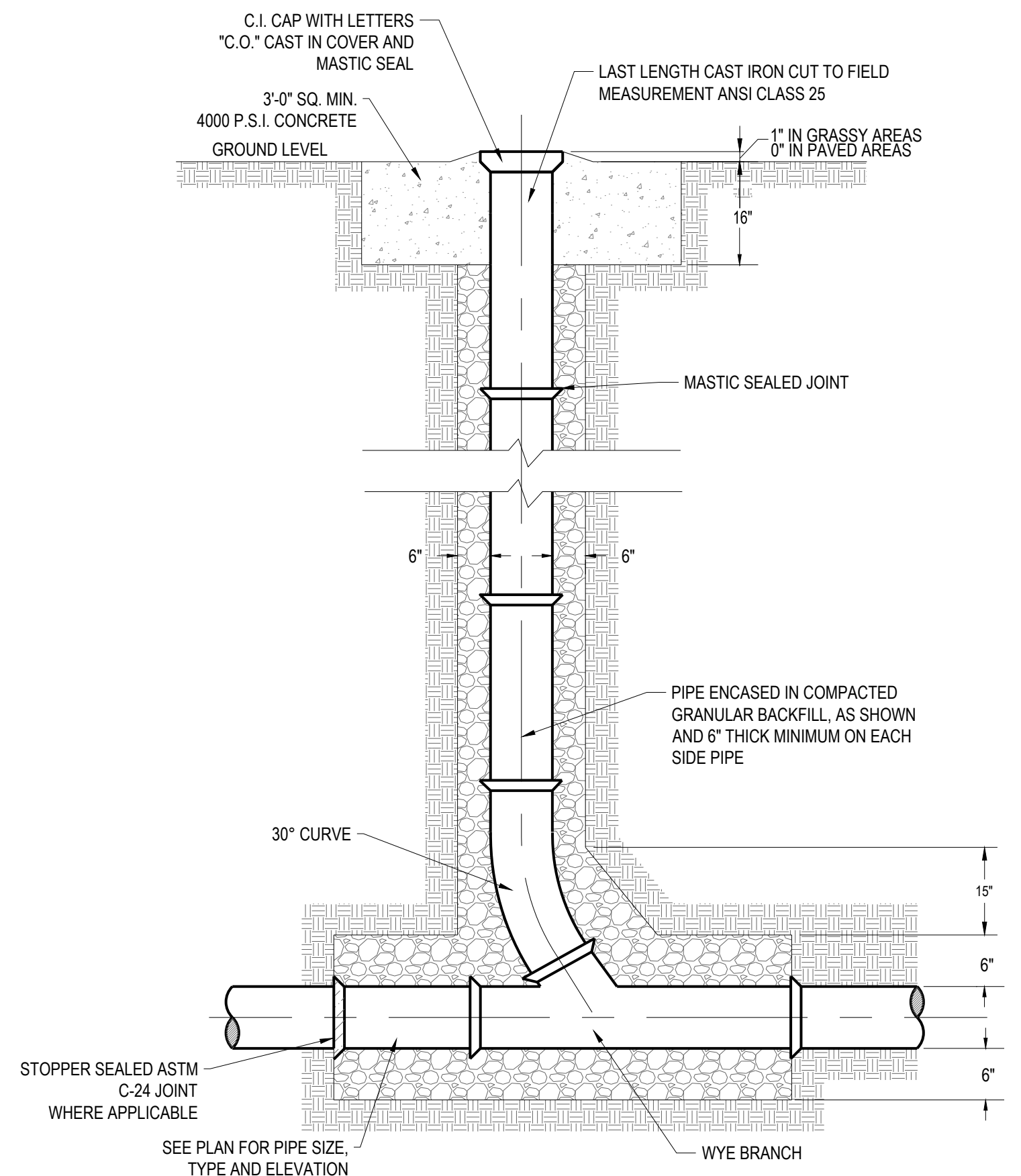


NOTE: FOR CONCRETE COLLARS ON FLAT TOP STRUCTURES, COLLARS SHALL BE PINNED TO THE TOP WITH TWO #4 BARS (ONE ON EACH SIDE) IN THE DIRECTION OF TRAFFIC.

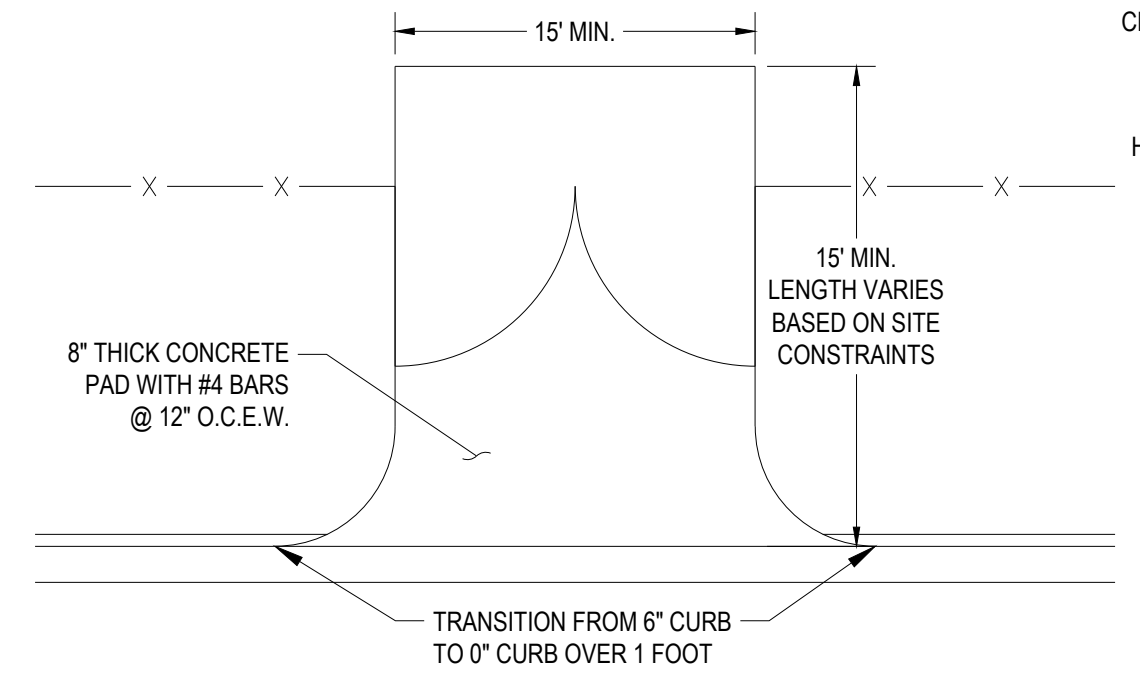
CONCRETE COLLAR DETAIL FOR 48" DIA. STRUCTURES (FOR USE ON CONE SECTION WITH 24" OR 36" OPENINGS)
NOT TO SCALE



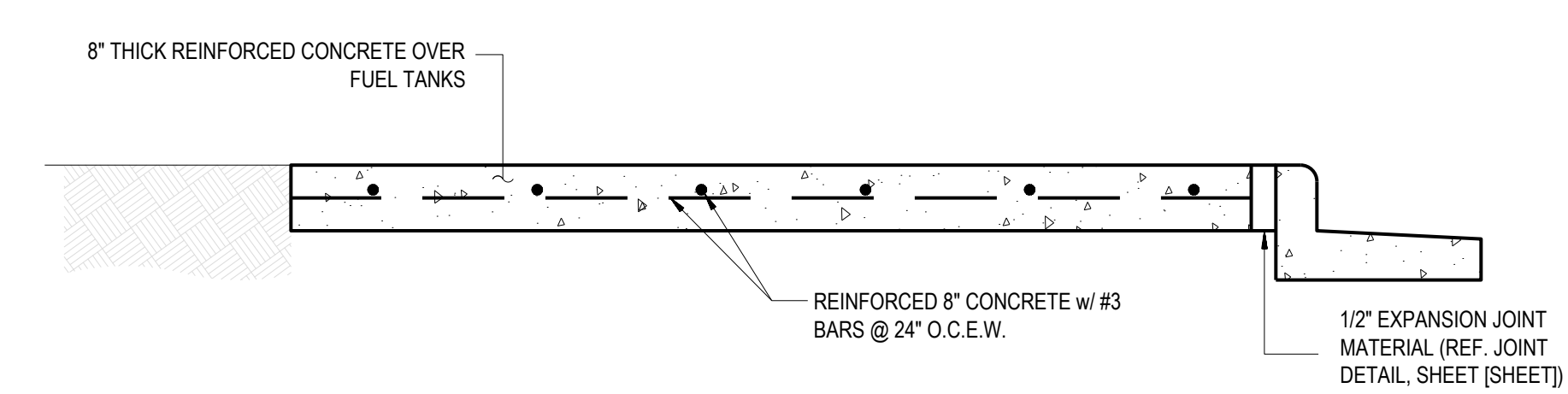
CONCRETE TO ASPHALT STANDARD PAVING HEADER
NOT TO SCALE



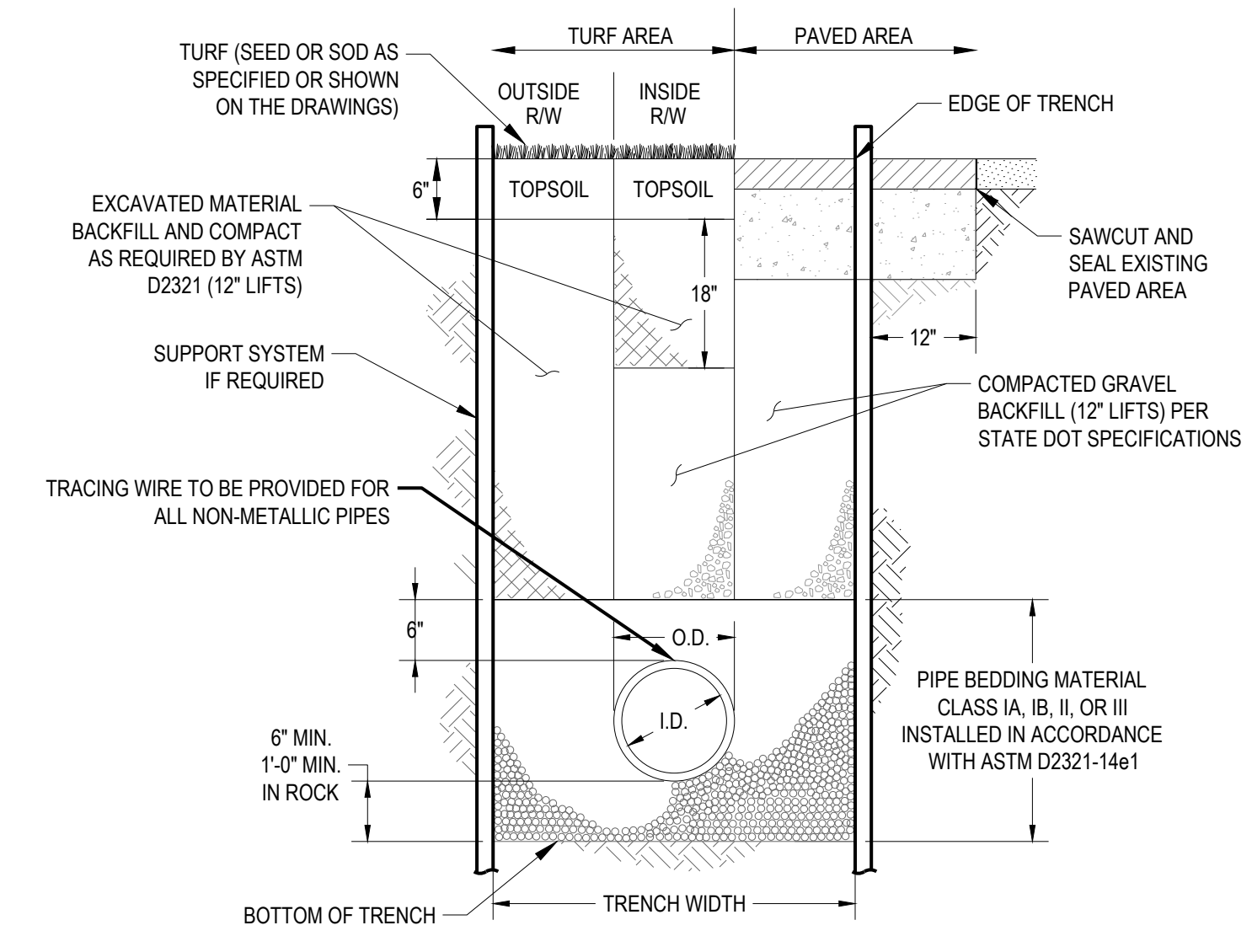
CLEAN OUT
NOT TO SCALE



NOTE:
1. MAXIMUM PAD SLOPE NOT TO EXCEED 4:1 IN THE LONGITUDINAL DIRECTION OR 4% CROSS-SLOPE.
2. PROVIDE GATE IN CHAIN-LINK FENCE AS NEEDED. GATE SHALL SWING OPEN TOWARDS THE PARKING LOT.
3. PROVIDE ROLLED DROP CURB AT PUSH PAD SO AS TO PROVIDE A SMOOTH TRANSITION FROM THE PARKING LOT TO THE PAD FOR THE SNOW PLOW.
4. REFER TO ARCHITECTURAL PLANS FOR FENCE/GATE DETAILS.



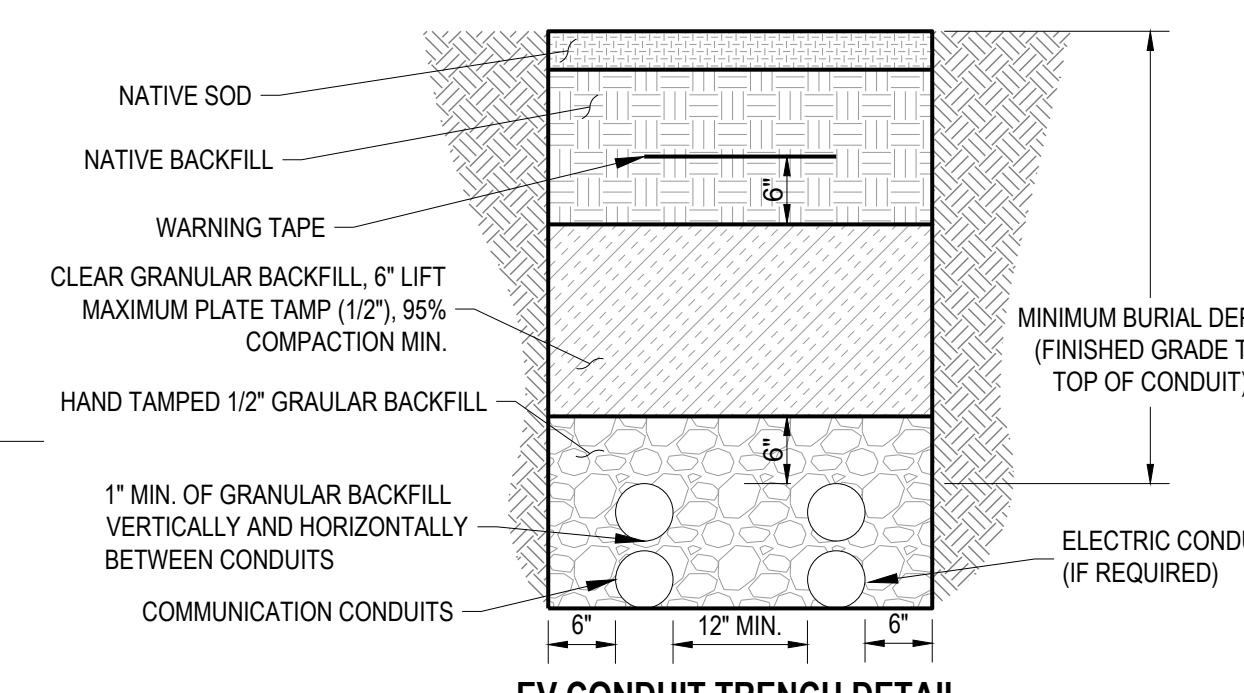
TYPICAL SECTION THROUGH PAVING OVER FUEL TANKS
NOT TO SCALE



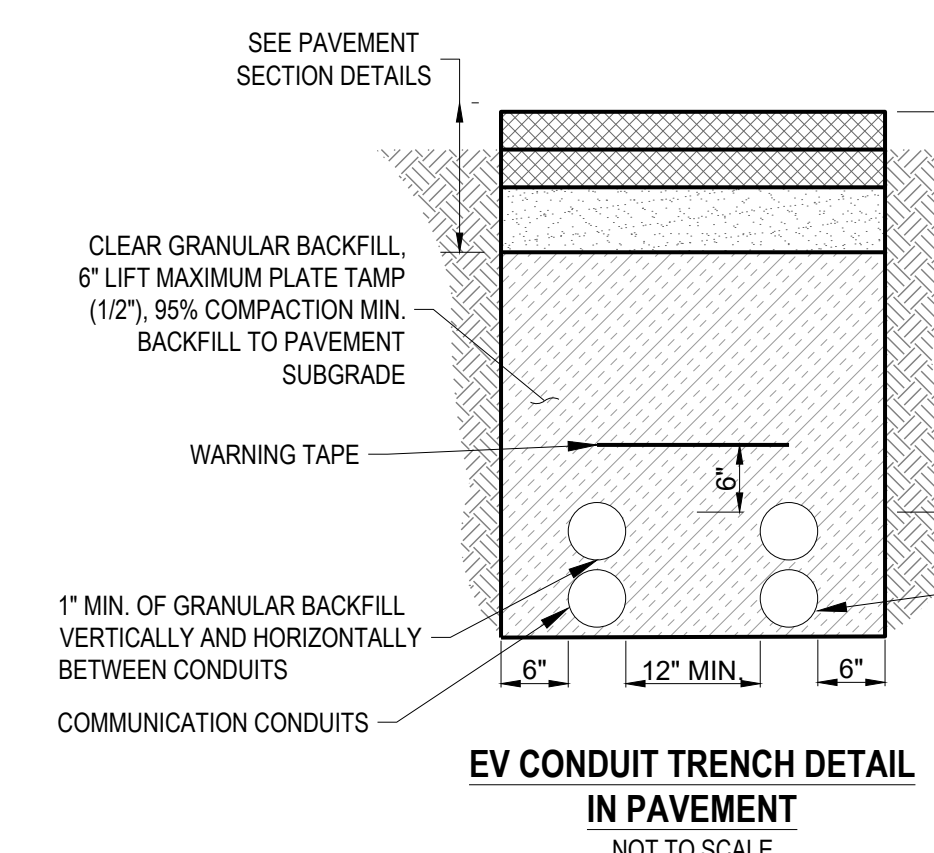
PIPE OUTSIDE DIAMETER	TRENCH WIDTH
LESS THAN OR EQUAL TO 15"	O.D. + 16"
GREATER THAN 15"	(O.D. X 1.25) + 12"

TRENCH WIDTH MAY BE LESS IF APPROVED BY THE ENGINEER. TRENCH WIDTH TO BE GREATER IN POOR OR UNSTABLE SOILS AS DETERMINED BY THE ENGINEER (REF. ASTM 2321).

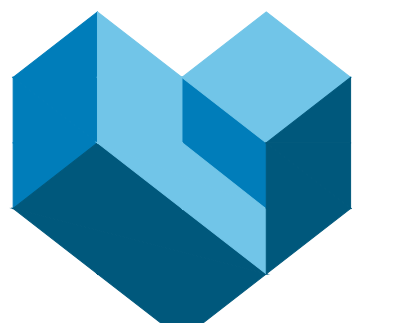
TRENCH BACKFILL
NOT TO SCALE



NOTE:
1. BURIAL DEPTHS ARE REQUIRED TO BE 2'-0" MIN. OR 6" BELOW FROST DEPTH PER LOCAL JURISDICTION.
2. EXACT NUMBER OF CONDUIT SHALL BE DETERMINED BY THE ELECTRICAL PLANS.
3. REFER TO ELECTRICAL PLANS FOR CONDUIT ROUTES.
4. IF NO ELECTRICAL CONDUIT IS REQUIRED, CENTER COMMUNICATIONS CONDUIT IN TRENCH.



NOTE:
1. PAVEMENT SECTION DETAILS TO COMPLY WITH GEOTECH REPORT SPECIFICATIONS.
2. BURIAL DEPTHS ARE REQUIRED TO BE 2'-0" MIN. OR 6" BELOW FROST DEPTH PER LOCAL JURISDICTION.
3. EXACT NUMBER OF CONDUIT SHALL BE DETERMINED BY THE ELECTRICAL PLANS.
4. REFER TO ELECTRICAL PLANS FOR CONDUIT ROUTES.
5. IF NO ELECTRICAL CONDUIT IS REQUIRED, CENTER COMMUNICATIONS CONDUIT IN TRENCH.



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12/18/2023

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43118

Revisions / Submissions

ID	Description	Date

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Project Number: 759267
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Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

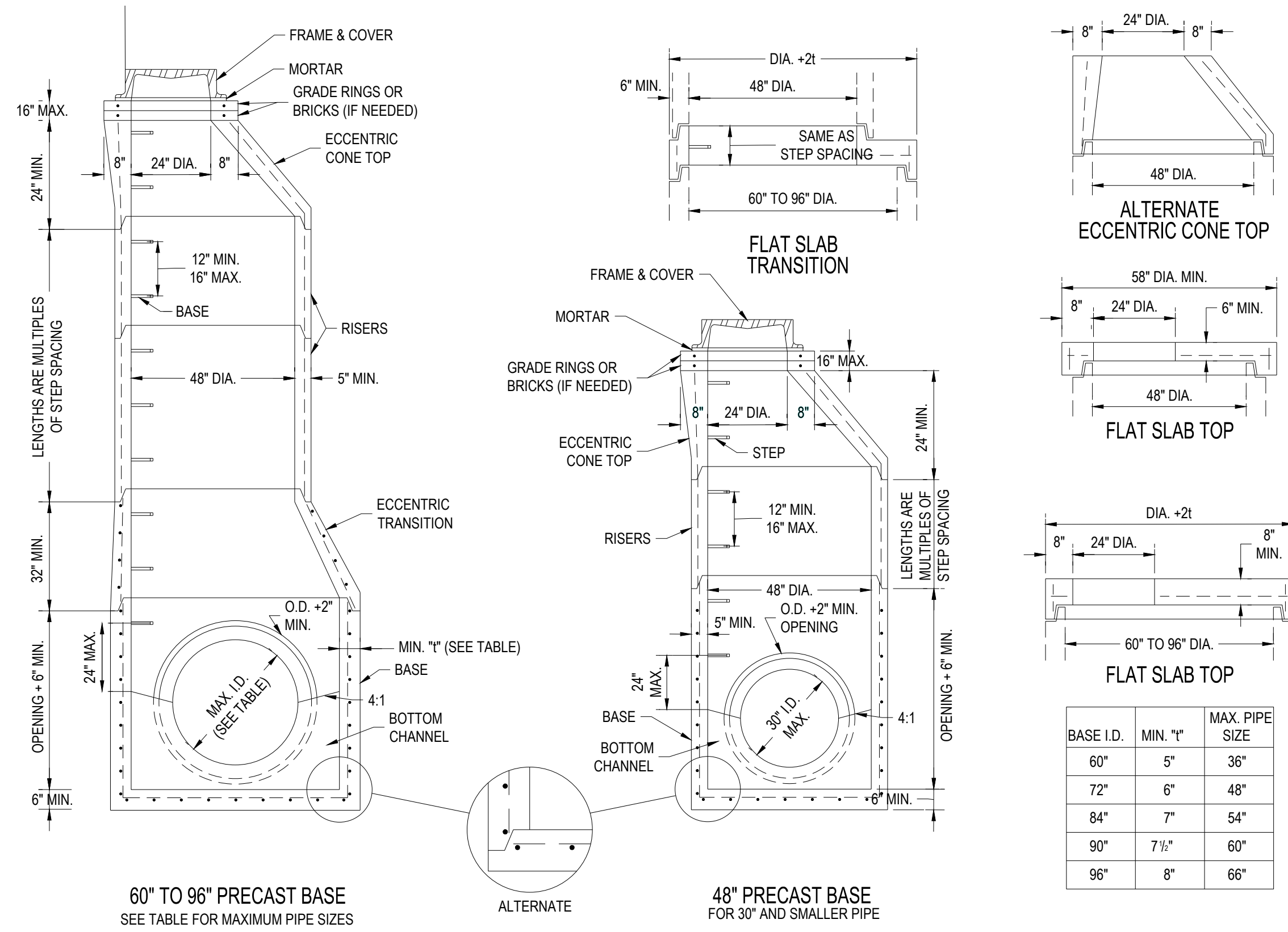
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CONSTRUCTION DETAILS

C9.1

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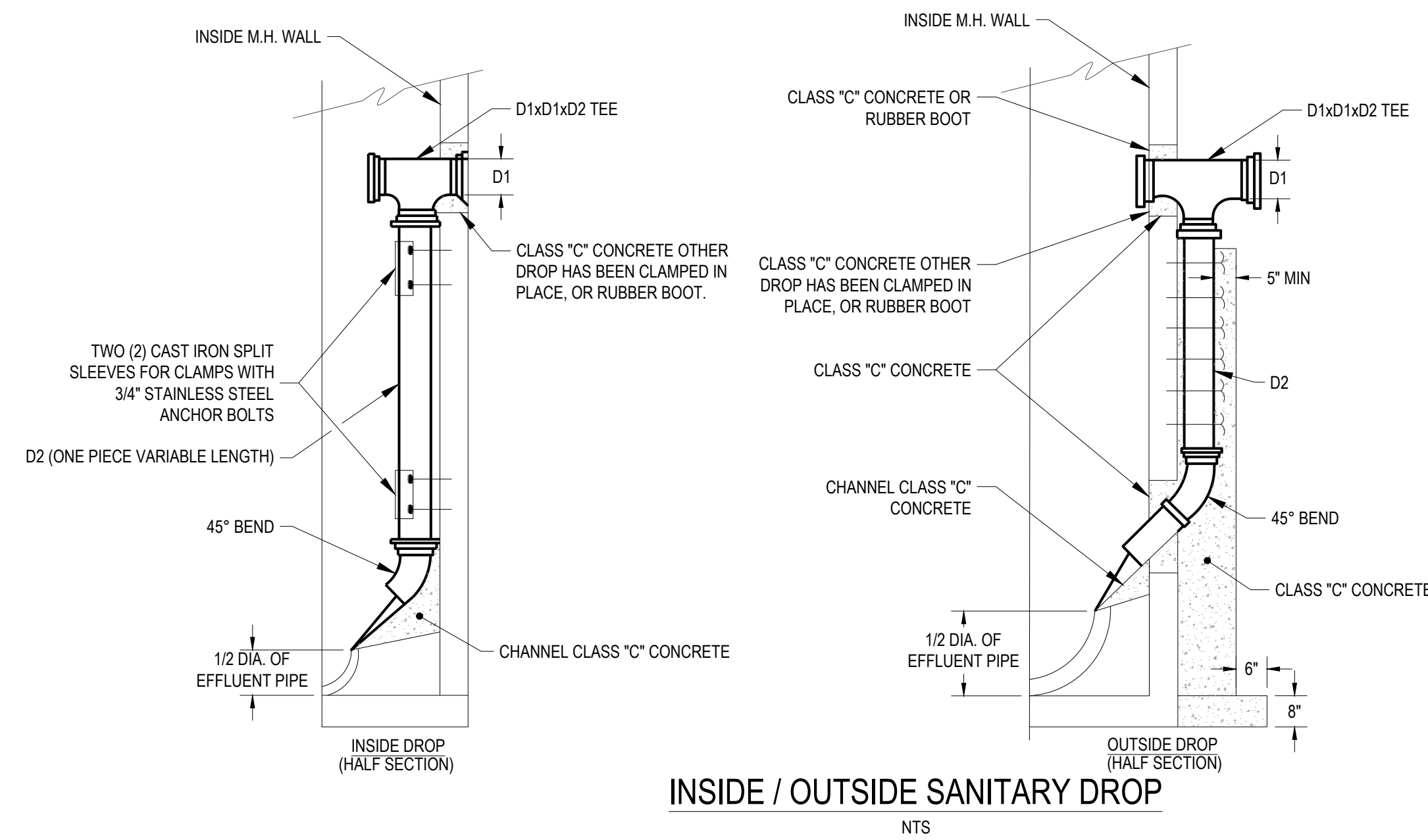
NOTES:

- SECTIONS OF THE PRECAST MANHOLE SHALL BE CAST AND ASSEMBLED WITH EITHER ALL TONGUE OR ALL GROOVE ENDS UP. LIFT HOLES MAY BE PROVIDED IN EACH SECTION FOR HANDLING.
- TOP AND TRANSITION (OR REDUCER) SECTIONS MAY BE EITHER ECCENTRIC CONE, CONCENTRIC CONE, OR FLAT SLAB.
- BASES FOR MANHOLES ARE SHOWN WITH MONOLITHIC FLOOR AND RISER WHICH MAY BE CAST IN ONE OR TWO OPERATIONS. A PERMISSIBLE ALTERNATE IS TO CAST AND SHIP THE FLOOR AND BARREL SEPARATELY. OPENINGS FOR INLET AND OUTLET PIPE SHALL BE PROVIDED EITHER WHEN THE UNIT IS CAST OR LATER, TO MEET PROJECT REQUIREMENTS. BOTTOM CHANNELS MAY BE FORMED OF CONCRETE PRECAST IN THE BASE OR BY FIELD CONSTRUCTION. BASES MAY ALSO BE POURED IN PLACE. ALL INLETS AND OUTLETS ARE TO BE IDENTIFIED.
- OPENINGS IN RISER SECTIONS FOR 18" AND SMALLER INLET PIPES SHALL BE PREFABRICATED. FLEXIBLE CONNECTIONS SHALL BE PROVIDED FOR SANITARY AND COMBINED SEWERS.
- JOINT SEAL BETWEEN PRECAST MANHOLE SECTIONS AND PIPES SHALL BE RESILIENT AND FLEXIBLE GASKET JOINTS PER ASTM C-923 OR LATEST EDITION.
- O-RING JOINT BETWEEN MANHOLE SECTIONS SHALL BE FLEXIBLE BUTYL RUBBER SEAL PER ASTM C-990.
- PRECAST MANHOLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-478. CONCRETE SHALL BE 4000 PSI.
- SEAL LIFT HOLES WITH NON-SHRINK GROUT.
- FRAME AND COVER:
1. FRAME WITH SOLID COVER USE EAST JORDAN 1710A, WITH APPROPRIATE LETTERING OR APPROVED OTHER.
2. FRAME WITH GRATE USE EAST JORDAN 1710M OR APPROVED OTHER.



PRECAST CONCRETE MANHOLE

NTS



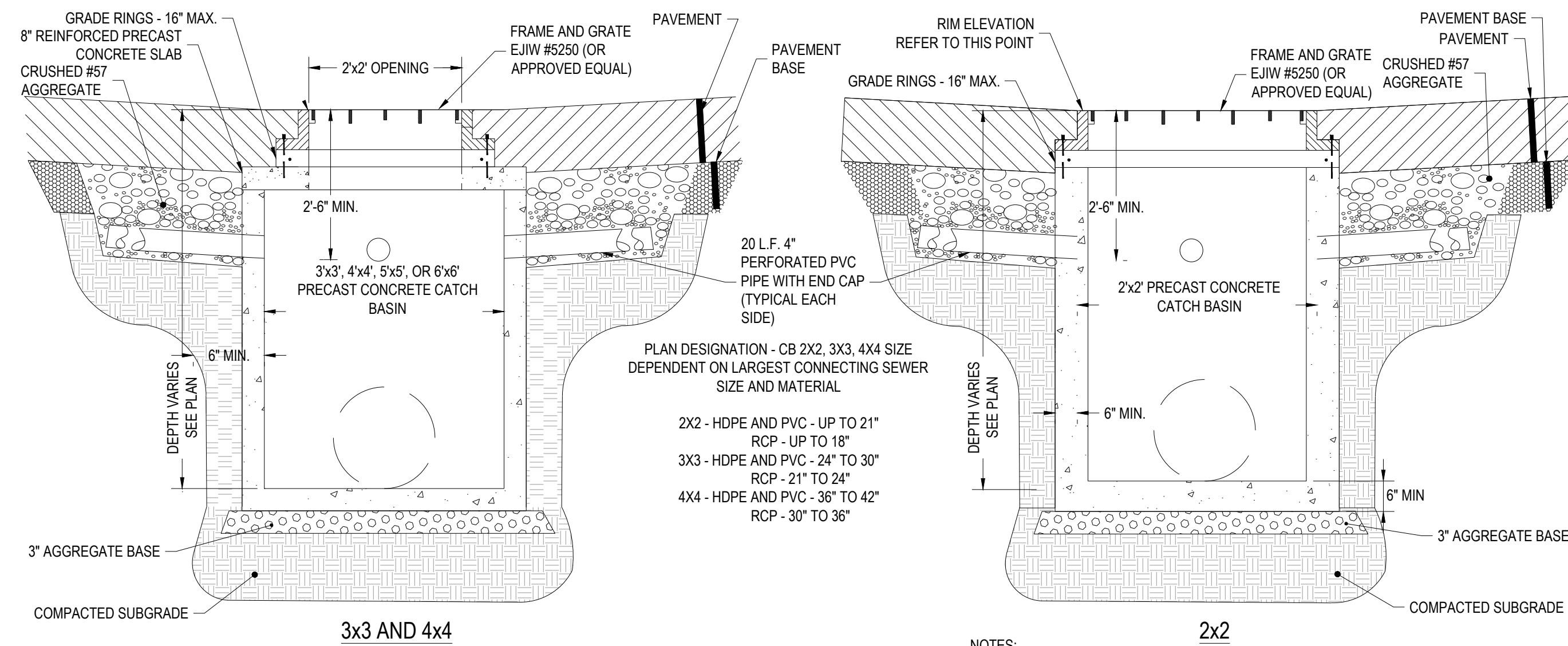
INSIDE / OUTSIDE SANITARY DROP

NTS

NOTES:

- HEIGHT OF DROP IS TO BE AS SHOWN ON THE PLANS OR WILL BE DETERMINED AT THE TIME OF CONSTRUCTION.
- WHERE CALLED FOR, AND UNLESS OTHERWISE REQUIRED BY THE PLANS, THE OUTSIDE DROP WILL BE CONSTRUCTED WITH NEW MANHOLES.
- MATERIALS FOR THE TEE, DROP PIPE AND BEND SHALL BE OF THE SAME MATERIAL AS THE UPSTREAM PIPE, UNLESS OTHERWISE DIRECTED BY THE LOCAL AUTHORITY HAVING JURISDICTION.
- OUTSIDE DROP PIPES REQUIRE A 5" THICK (MINIMUM) CLASS "C" CONCRETE ENCASEMENT ON THREE SIDES OF PIPE

PIPE DIAMETER	D1	D2
4"	4"	4"
6"	6"	6"
8"	6"	6"
10"	8"	8"
12"	8"	8"
15"	10"	10"
18"	10"	10"
21"	12"	12"
24"	12"	12"

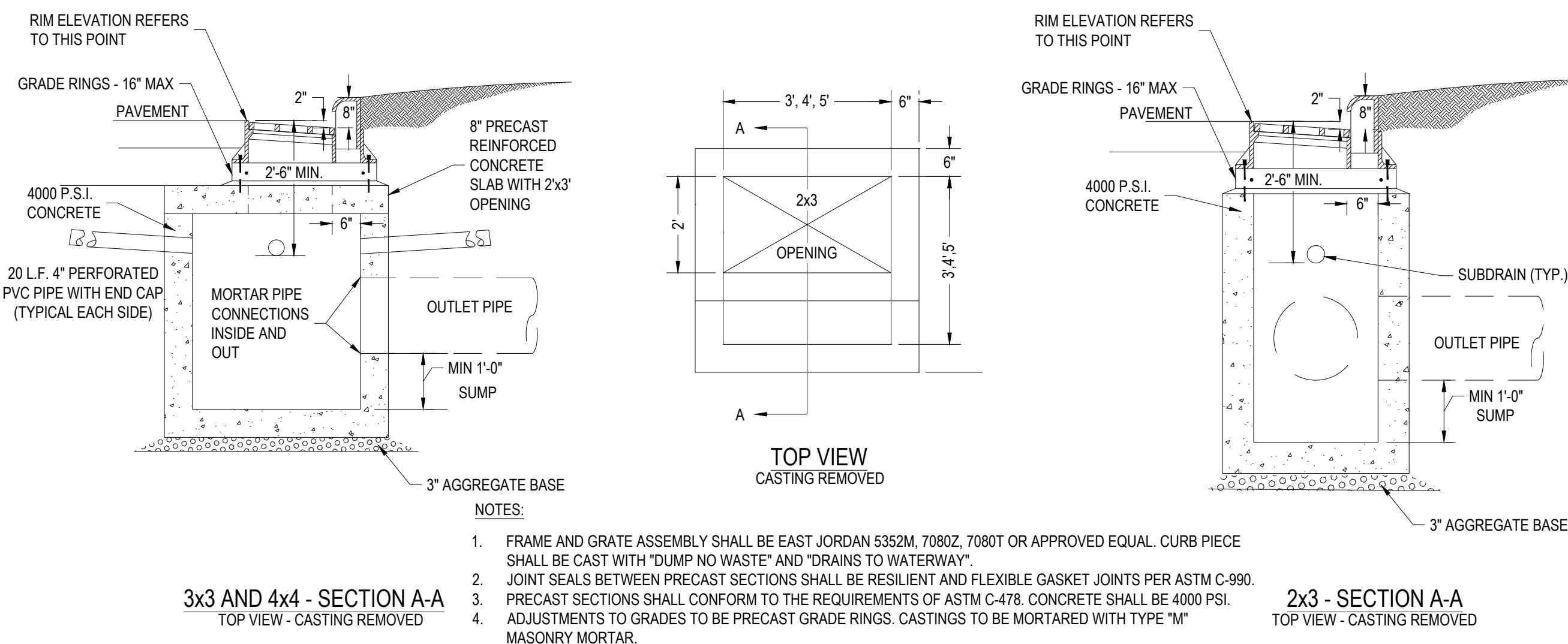


PRECAST CATCH BASIN

NTS

NOTES:

- TOP OF GRATES SHALL BE CAST WITH "DUMP NO WASTE" AND "DRAINS TO WATERWAY". JOINTS BETWEEN SECTIONS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-990 OR LATEST EDITION. PRECAST SECTIONS SHALL CONFORM TO THE REQUIREMENTS TO ASTM C-478. CONCRETE SHALL BE 4000 PSI.
- ADJUSTMENT TO GRADES TO BE PRECAST GRADE RINGS. CASTINGS TO BE MORTARED WITH TYPE "M" MASONRY MORTAR.



PRECAST CURB INLET

NTS

- FRAME AND GRATE ASSEMBLY SHALL BE EAST JORDAN 5352M, 7080Z, 7080T OR APPROVED EQUAL. CURB PIECE SHALL BE CAST WITH "DUMP NO WASTE" AND "DRAINS TO WATERWAY".
- JOINT SEALS BETWEEN PRECAST SECTIONS SHALL BE RESILIENT AND FLEXIBLE GASKET JOINTS PER ASTM C-990.
- PRECAST SECTIONS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-478. CONCRETE SHALL BE 4000 PSI.
- ADJUSTMENTS TO GRADES TO BE PRECAST GRADE RINGS. CASTINGS TO BE MORTARED WITH TYPE "M" MASONRY MORTAR.



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12/18/2023

LOVES TRAVEL STOP

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66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions

ID	Description	Date

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Project Number: 759267

Scale: N/A

Drawn By: FAR

Checked By: JTK

Date: 12/19/2023

Issue: OUT TO BID

Drawing Title:

**CONSTRUCTION
DETAILS**

C9.2



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LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions		
ID	Description	Date
1	REV 1	01/03/2024
4	ADDENDUM 4	02/23/2024

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Scale: N/A
Drawn By: CDR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title: **CONSTRUCTION DETAILS**

C9.3

FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



GENERAL CONSTRUCTION REQUIREMENTS

- PRESSURE TESTING OF WATER LINES AND SEWER FORCE MAINS SHALL BE IN ACCORDANCE WITH AWWA C600-87. TESTING AT 1-1/2 TIMES EXISTING PRESSURE AND/OR WORKING PRESSURE OR MINIMUM OF 100PSI/MAX. OF THE PRESSURE RATING OF THE PIPE. PRESSURE TESTING MUST BE DONE WITHIN 2 WORKING WEEKS ONCE LINE IS IN PLACE.
- THE CONTRACTOR WILL CHLORINATE THE INSTALLED WATER LINE AT RECOMMENDED AWWA 651 STANDARDS, FOR A MINIMUM OF 24 HOURS, AT 50 P.P.M.
- THE DISTRICT WILL TAKE REPRESENTATIVE SAMPLES OF THE WATER IN THE LINES, AND HAVE AN APPROVED LABORATORY MAKE THE BACTERIOLOGICAL EXAMINATIONS OF THE SAMPLE TO ESTABLISH THE ACCEPTABILITY OF THAT PORTION OF THE SYSTEM PRIOR TO ANY TURN ON OF SERVICE TO ANY FACILITY. THERE WILL BE NO EXCEPTION TO THIS STANDARD.
- ALL MAIN LINE VALVES & FLUSH-OUT ASSEMBLIES SHALL BE MARKED BY A WHITE FIBERGLASS MARKER AS PER THE WATER DISTRICTS REQUIREMENTS.
- LOCATION OF THE PROPOSED SERVICE TAPS SHALL BE FIELD LOCATED BY THE OWNER PRIOR TO CONSTRUCTION OR AT TIME OF CONSTRUCTION.
- ALL MATERIAL AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BELMONT COUNTY SPECIFICATIONS AS OUTLINED IN STANDARD DETAILS MANUAL.
- ALL PARTS, LABOR, EQUIPMENT, AND MISC. EXPENSES BY THE BELMONT COUNTY SANITARY SEWER DISTRICT WILL BE INVOICED TO THE DEVELOPER.
- CONTRACTOR IS RESPONSIBLE FOR ALL WORK PERFORMED FOR THE FIRST YEAR OF CUSTOMER SERVICE.
- ALL PRESSURE TESTING STARTS BEFORE NOON.
- AS-BUILT DRAWINGS (MAPS) MUST BE PROVIDED TO BCWS WITHIN 3 MONTHS OF THE BACTERIA TEST, WHICH MARKS THE WATER LINE COMPLETION DATE. FOR 6" AND LARGER LINES AND WITHIN ONE MONTH FOR 21/4" LINES.
- AS-BUILT FOR 6" & LARGER LINES ARE REQUIRED TO HAVE GPS POINT WHEN SUBMITTED FOR APPROVAL. THEY ARE TO POINT OUT THE FOLLOWING HYDRANT FITTINGS: CURB BOXES, METER WELLS, MAN HOLES, CLEAN OUTS, VALVES, AND WATER OR SEWER LINES IN 100FT. INCREMENTS.

BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

2/15/24 3A

GENERAL CONSTRUCTION REQUIREMENTS

- A 16FT. MIN. LENGTH OF WATERLINE SHALL BE PROVIDED ON EACH SIDE OF ANY MAINLINE FITTING, OR THE JOINT SHALL BE RESTRAINED WITH BELL RESTRAINT FITTINGS.
- ALL SERVICE CONNECTIONS MADE TO EXISTING LINES WILL BE MADE BY THE DISTRICT. THE CONNECTION SHALL BE MADE WET USING A TAPPING TEE AND VALVE. IF A LINE TO BE CONNECTED IS THE SAME AS THE EXISTING LINE, 2" CONNECTED TO 2" OR 6" CONNECTED TO 6". THE CONNECTION WILL BE MADE DRY. EXCEPTIONS MAY BE MADE DUE TO UNFORESEEN FIELD CIRCUMSTANCES.
- MAGNETIC DETECTABLE TRACER TAPE SHALL BE INSTALLED AT 18 INCHES DEEP WITH ALL PVC PIPE.
- TRACE WIRE TO BE INSTALLED ON ALL WATER & SEWER LINES.
 - OPEN-TRENCH INSTALLATION: DIRECT BURIAL #12 AWG SOLID (0.0808" DIA.), STEEL CORE SOFT DRAWN TRACER WIRE, 250# AVERAGE TENSILE BREAK LOAD, 30 MIL HIGH MOLECULAR-HIGH DENSITY POLYETHYLENE JACKET COMPLYING WITH ASTM-D-1248, 30 VOLT RATING, 45 MIL JACKET WHEN DIRECTIONAL BORED.
 - CONNECTORS: SPLICES ALONG THE CONTINUOUS RUN OF TRACE WIRE FOR REPAIR OF A WIRE BREAK OR REPLACEMENT OF FAILED SEGMENT OF WIRE SHALL USE 3M BRAND DBR DIRECT BURY SPLICE KIT OR APPROVED EQUAL.
 - INSTALLATION: TRACE WIRE ACCESS POINT SHALL BE ACCESSIBLE AT ALL NEW WATER VALVE BOXES, WATER METER BOXES, BLOWOFFS, ARVs, FIRE HYDRANTS AND MANHOLES.
 - TESTING REQUIREMENTS: CONTRACTOR SHALL PERFORM A CONTINUITY TEST ON ALL TRACE WIRE IN THE PRESENCE OF BCWS. IF THE TRACE WIRE IS FOUND TO BE NOT CONTINUOUS AFTER TESTING, CONTRACTOR SHALL REPAIR OR REPLACE THE FAILED SEGMENT OF WIRE.
- THE CONTRACTOR WILL SCHEDULE WITH THE DISTRICT APPROXIMATE TIMES FOR THE INSPECTION OF LINES, FITTINGS & ETC., PRIOR TO BACKFILLING. THE COUNTY SHALL BE GIVEN A MINIMUM 48 HOURS OF NOTICE WHEN AN INSPECTION IS REQUIRED.
- EXISTING MAINLINE VALVES WILL ONLY BE OPERATED BY DISTRICT PERSONNEL.
- A MINIMUM OF 24 HOURS NOTICE FOR ANY INSTALLATIONS BY CONTRACTORS THAT MAY DISRUPT WATER SERVICE TO EXISTING WATER CUSTOMERS. THE NOTICE SHALL BE MADE THROUGH THE DISTRICTS OFFICE.
- ALL SERVICE CONNECTIONS INSTALLED WILL BE STAKED AT THE TIME OF CONSTRUCTION WITH IRON "TEE BAR STAKES" PER DISTRICT REQUIREMENTS.
- CHANGES IN GRADE AND ALIGNMENT TO BE OBTAINED BY DEFLECTIONS AT PIPE JOINTS SHALL BE NO GREATER THAN RECOMMENDED BY THE MANUFACTURER OF THE WATER LINE.

BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

02/15/24 3

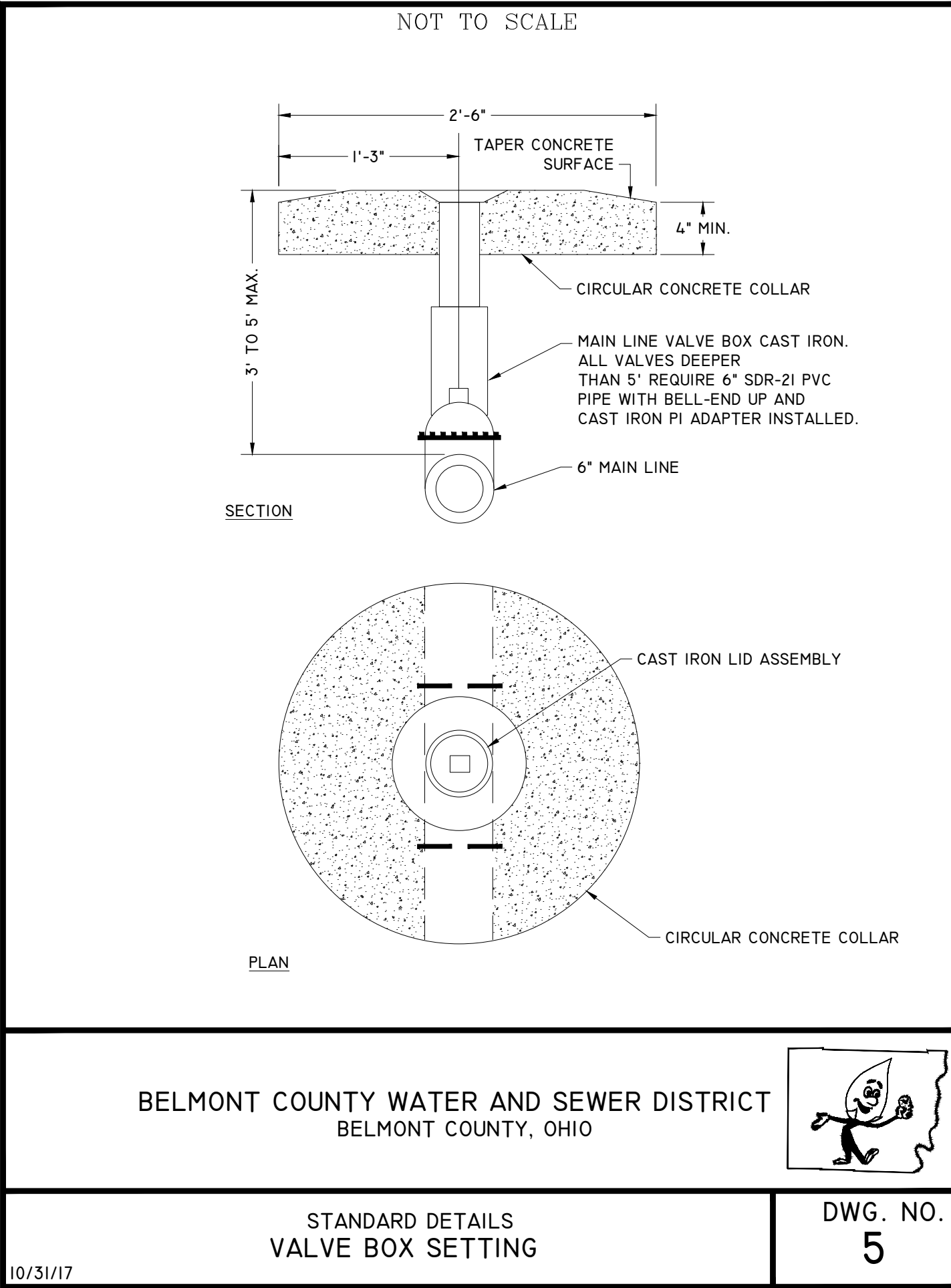
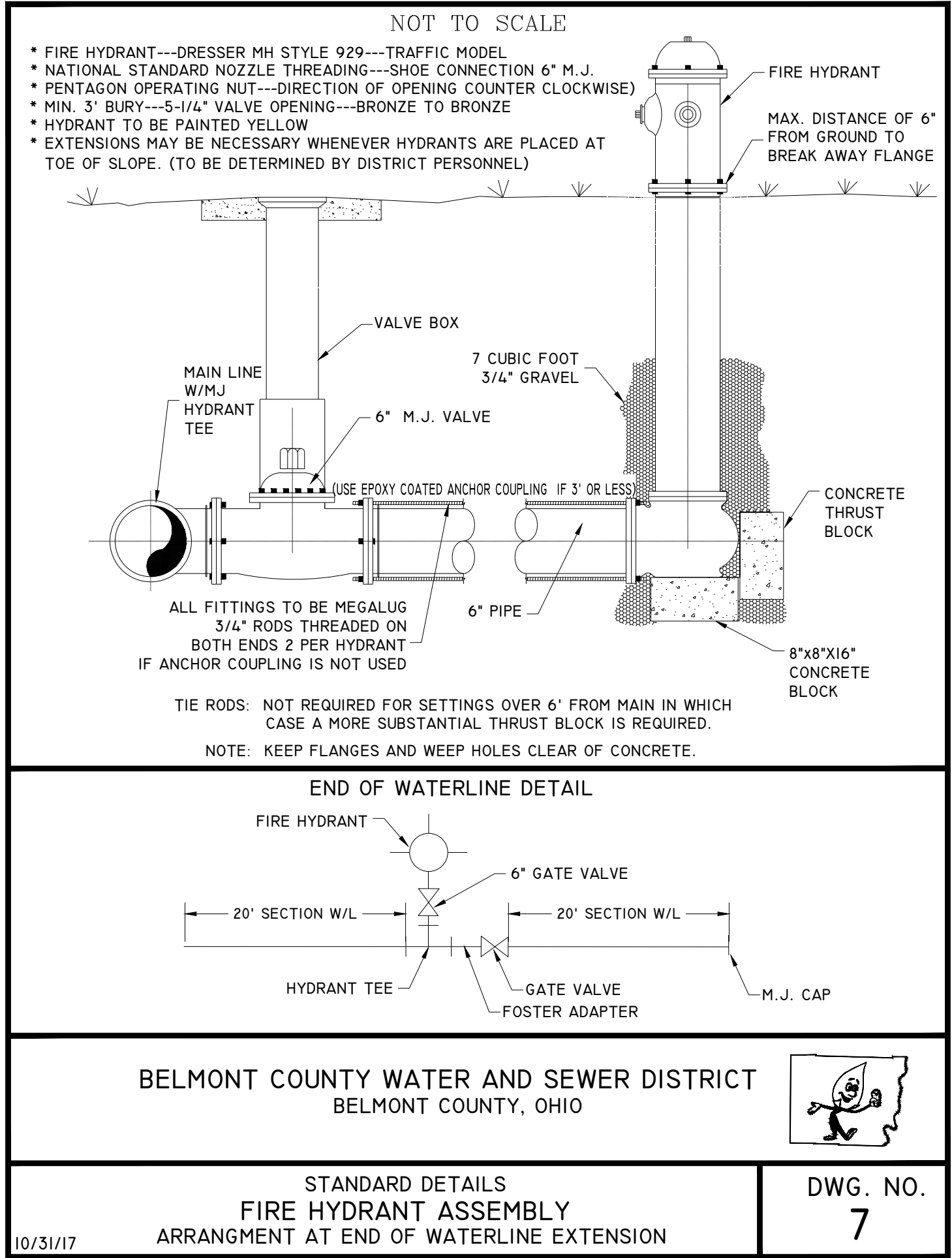
GENERAL CONSTRUCTION REQUIREMENTS

- DETAIL PLANS OF PLANNED WATER LINE CONSTRUCTION WILL BE SUBMITTED TO THE BELMONT COUNTY WATER AND SEWER DISTRICT FOR APPROVAL BY THE ENGINEER ASSOCIATE, PRIOR TO CONSTRUCTION. A PRECONSTRUCTION MEETING WILL BE REQUIRED BEFORE START OF CONST.
- SIGNED WATER APPLICATIONS & APPLICABLE FEES MUST BE RECEIVED PRIOR TO CONSTRUCTION.
- ALL WATERLINE'S SHALL BE C900 (DR-14) IN HIGH PRESSURE AREAS DUCTILE IRON CLASS 50 OR 52 WILL BE REQUIRED BY THE DISTRICT. (ALL DUCTILE IRON PIPE TO BE POLYETHYLENE WRAPPED AS PER AWWA C105/A21.5) PIPE SIZES TO BE 3/4"CTS, 1"CTS, 2", 4", 6", 8", 10", 12" C-900 (DR 14) (3/4" OR 1" TO BE TYPE-K SOFT COPPER IF LINE PRESSURE IS GREATER THAN 200 PSI) NO PIPE LENGTHS UNDER 4'.
- 2" AND LARGER PRIVATE SERVICE LINES THAT CROSS UNDER A ROADWAY WILL REQUIRE AN ADDITIONAL VALVE AT THE RIGHT-OF-WAY ON THE OPPOSITE SIDE OF MAINLINE.
- RESIDENTIAL PVC SEWER LINE WILL BE A MINIMUM OF 4 INCH SDR-35. ALL SEWER PIPE TO BE GASKETED.
- SEWER FORCE MAIN WILL BE A MINIMUM OF 1 1/2" OR 2" POLYETHYLENE CLASS 200 OR 2", 4", 6" OR 8" SDR-21 CL-200 PVC PIPE, AND GREEN IN COLOR. GRAVITY SEWER LINES SHALL BE AIR AND DEFLECTION TESTED IN ACCORDANCE WITH TABLE 1 PAGE 29 OF THIS BOOKLET.
- ALL PIPE THAT IS INSTALLED SHALL BE BEDDED WITH A MINIMUM OF 6" ABOVE AND BELOW THE PIPE WITH GOOD TOP SOIL FREE OF STONE AND OTHER FOREIGN OBJECTS. OR CRUSH AND RUN STONE.
- WATERLINE SHALL BE INSTALLED WITH 4 FOOT MINIMUM COVER UNLESS OTHERWISE APPROVED.
- A MINIMUM VERTICAL CLEARANCE OF 18 INCHES SHALL BE PROVIDED BETWEEN THE PROPOSED WATER MAIN AND OTHER UTILITIES OR STRUCTURES, UNLESS OTHERWISE NOTED.
- A MINIMUM HORIZONTAL CLEARANCE OF 4 FEET SHALL BE PROVIDED BETWEEN THE PROPOSED WATER LINE AND OTHER UTILITIES OR STRUCTURES UNLESS OTHERWISE APPROVED.
- THE WATERLINE SHALL BE PLACED WITH A MINIMUM OF 10 FOOT HORIZONTAL SEPARATION FROM ANY SANITARY SEWERS OR WHEN THE WATER LINE CROSSES A SANITARY SEWER, THE WATER LINE SHALL BE A MINIMUM OF 18 INCHES ABOVE THE SEWER.
- SEWER LINES MUST CROSS UNDER ALL WATERLINES WITH A MINIMUM OF 18" OF CLEARANCE. IF NOT POSSIBLE THE BCSSD WILL PERMIT CROSSING OVER WITH STEEL CASING OR PLASTIC CASING AND CONCRETE. MINIMUM CASING LENGTH IS 10FT. AS PER NOTE ON SHT. #16
- ALL FITTINGS SHALL BE MECHANICAL JOINT (MJ), FUSION EPOXY COATED, DUCTILE IRON, RESTRAINED BY THE USE OF RESTRAINERS AND/OR MEGALUGS AND THRUST BLOCKED WITH CONCRETE AS REQUIRED. SEE SHT # 25 FOR THRUST BLOCKING DETAIL.

CONTINUES ON NEXT PAGE

BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

02/15/24 2



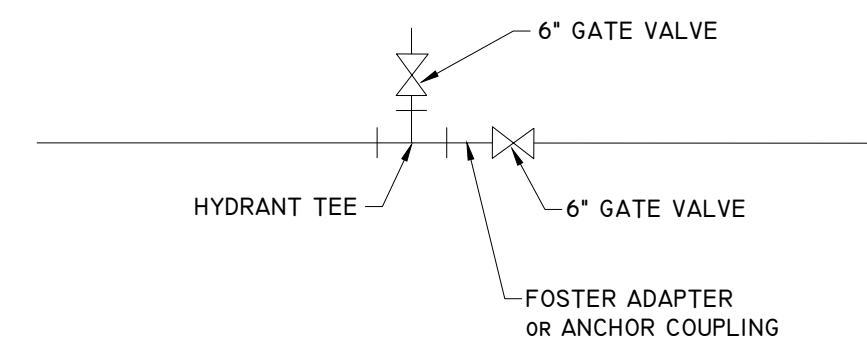
10

14

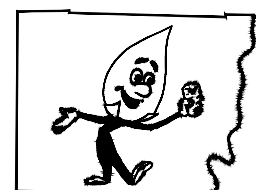
C:\P\AS-WATERSTANDARD DETAILS\DETAILS\CONSTRUCTION REQU_2015.dwg, 2/15/2024 9:07:50 AM

W:\PROJECTS\LOVES\759267 - St. Clairsville, OH - SR 149\03-CIVIL\PLAN\LOT-COMBINED\759267 SITE CONSTRUCTION DETAILS.dwg - 4/12/2024 - Ryan Ash

6" x 6" VALVE ARRANGMENT & HYDRANT TEE ON W/L



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

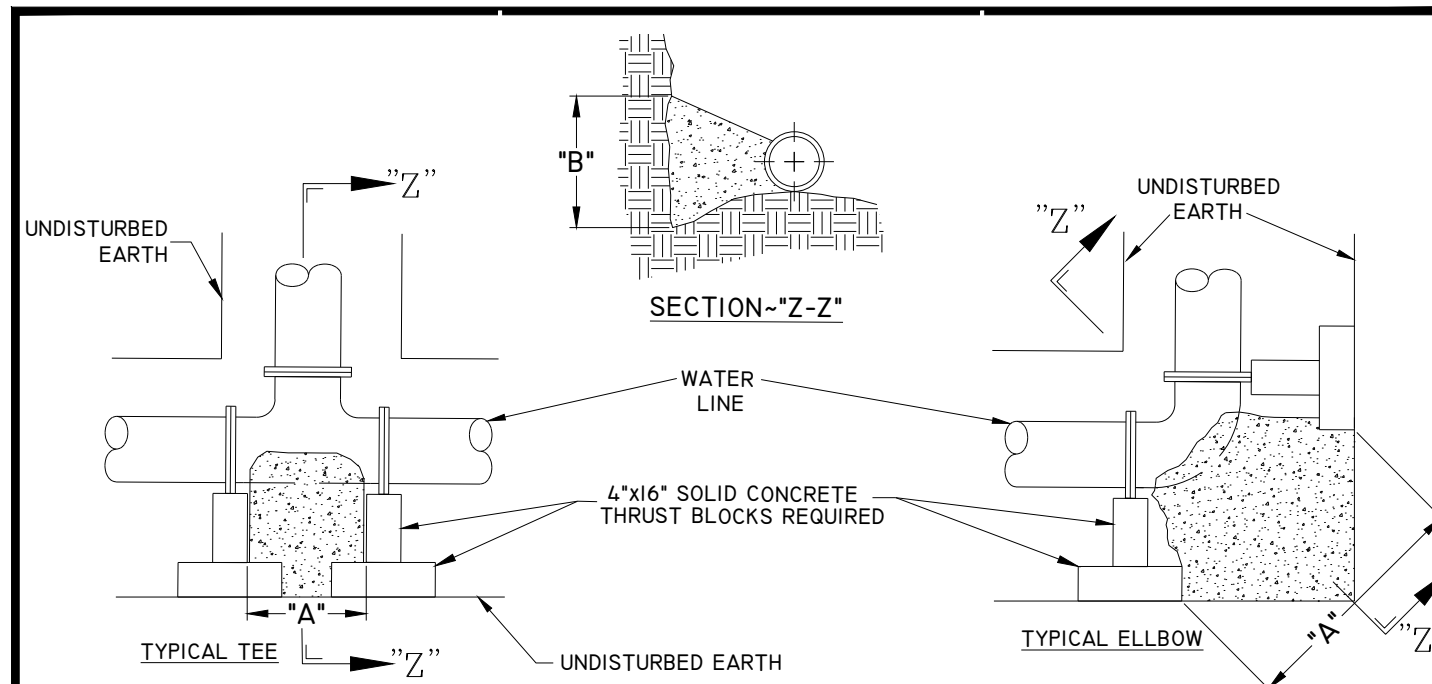


STANDARD DETAILS
6" X 6" VALVE ARRANGMENT

DWG. NO.
7A

9/25/17

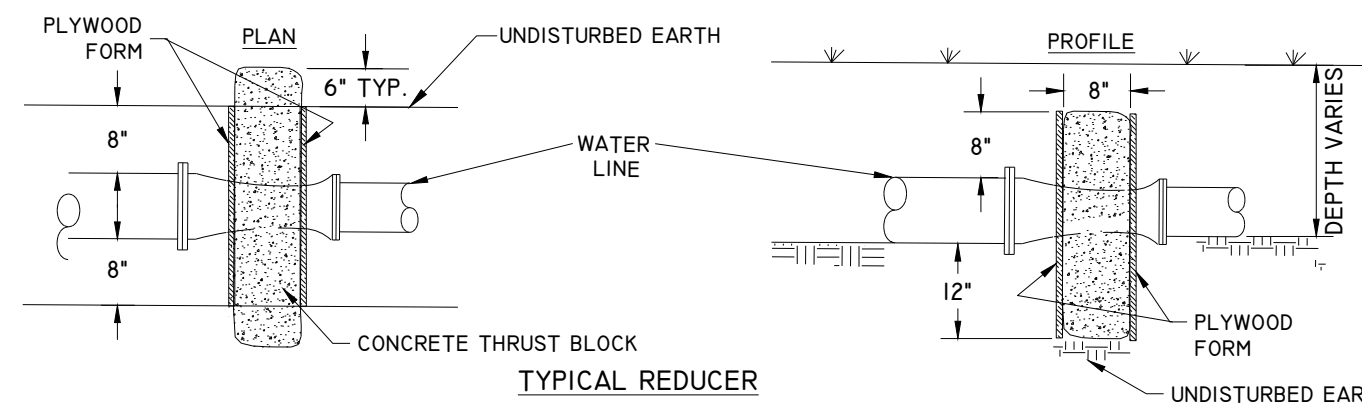
14A



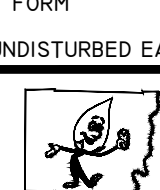
MINIMUM THRUST BLOCKING REQUIREMENTS

PIPE SIZE	90° BENDS		45° BENDS		22-1/2° BENDS		11-1/4° BENDS		TEES AND PLUGS	
	AREA 50 FT.	"A" "B"	AREA 50 FT.	"A" "B"	AREA 50 FT.	"A" "B"	AREA 50 FT.	"A" "B"	AREA 50 FT.	"A" "B"
4 INCH OR LESS	2.3	22" 15"	1.5	12" 18"	1.0	12" 12"	0.75	12" 9"	2.25	18" 18"
6 INCH	4.0	32" 18"	3.75	30" 18"	1.5	18" 12"	1.0	12" 12"	4.0	32" 18"
8 INCH	7.0	42" 24"	6.0	36" 24"	3.0	24" 12"	1.5	18" 12"	6.0	36" 24"
10 INCH	11.0	53" 30"	9.0	36" 30"	5.0	30" 24"	2.25	18" 18"	8.33	40" 30"
12 INCH	16.1	64" 36"	13.5	54" 36"	7.5	36" 30"	3.0	9" 36"	11.0	44" 36"
14 INCH	21.6	74" 42"	17.5	60" 42"	10.5	42" 36"	5.0	30" 24"	14.0	48" 42"
16 INCH	28.3	85" 48"	24.75	66" 54"	14.0	48" 42"	6.0	36" 24"	17.3	52" 48"

- NOTE'S:
1.) SOLID BLOCKS MAY BE USED UNDER FITTINGS AND ALONG SIDES OF THRUST BLOCKING TO HOLD CONCRETE IN PLACE. ALL CONCRETE MUST BE MIXED 4000# MINIMUM
2.) ALL FITTINGS TO BE WRAPPED WITH 8-MIL PLASTIC, LEAVING NO CONCRETE ON BOLTS



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



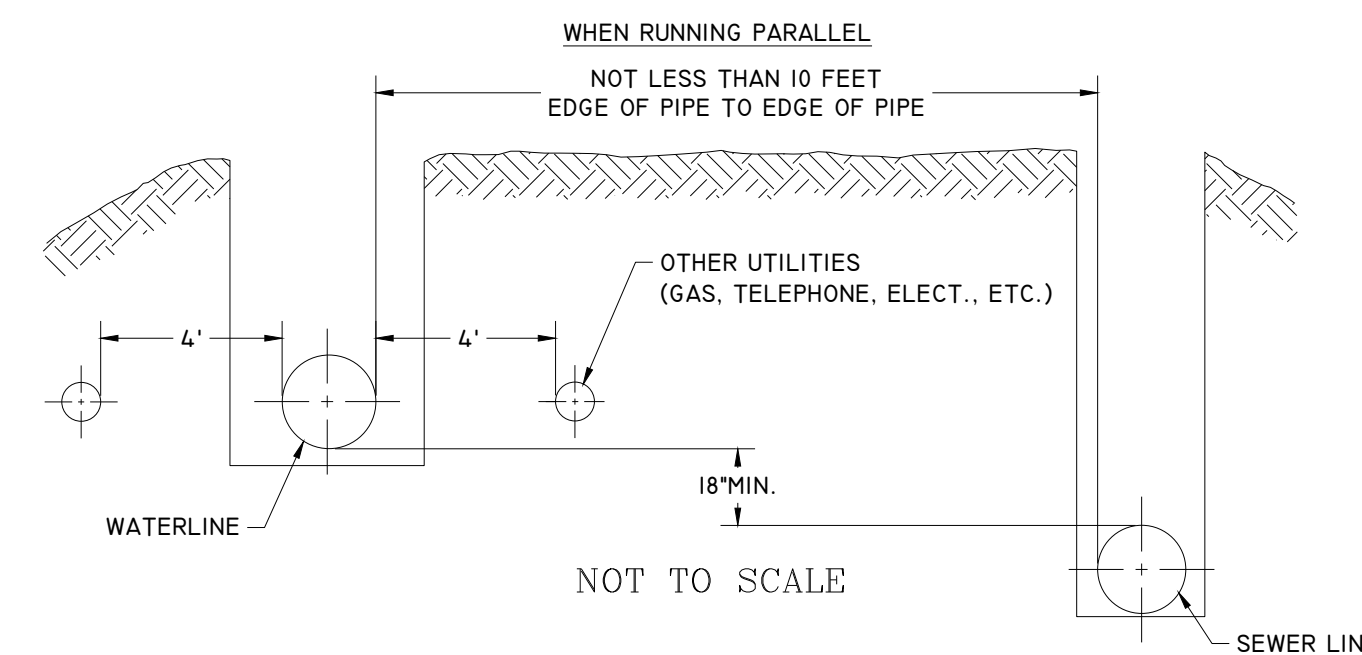
STANDARD DETAILS
THRUST BLOCKING

DWG. NO.
8

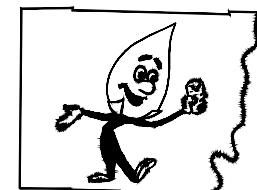
12/6/17

15

CROSSINGS
SEWERS CROSSING WATER MAINS SHALL BE LAID TO PROVIDE A MIN. VERTICAL DISTANCE OF 18" BELOW THE OUTSIDE OF THE WATER MAIN AND THE OUTSIDE OF THE SEWER. THIS SHALL BE THE CASE WHERE THE WATER MAIN IS EITHER ABOVE OR BELOW THE SEWER. THE CROSSING SHALL BE ARRANGED SO THAT THE SEWER JOINTS WILL BE EQUIDISTANT AND AS FAR AS POSSIBLE FROM THE WATER MAIN JOINTS. WHERE A WATER MAIN CROSSES UNDER A SEWER, ADEQUATE STRUCTURAL SUPPORT SHALL BE PROVIDED FOR THE SEWER TO MAINTAIN LINE AND GRADE. EITHER THE WATER MAIN OR THE SEWER LINE MUST BE ENCASED IN A WATERTIGHT CARRIER PIPE WHICH EXTENDS 10' ON BOTH SIDES OF THE CROSSING, MEASURED PERPENDICULAR TO THE MAIN. THE CARRIER PIPE SHALL BE MIN. 3/8" THICK STEEL OR ODOT ITEM 603 CONDUIT TYPE B, 707.42 WITH CONCRETE AS PER CONCRETE ENCASEMENT DETAIL ON DWG#10.
IF SEWER LINE IS A 4" SERVICE LINE CROSSING OVER OR UNDER A MAIN WATER LINE, THE CARRIER PIPE LENGTH MAY BE REDUCED TO 5' ON BOTH SIDES OF THE CROSSING.



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

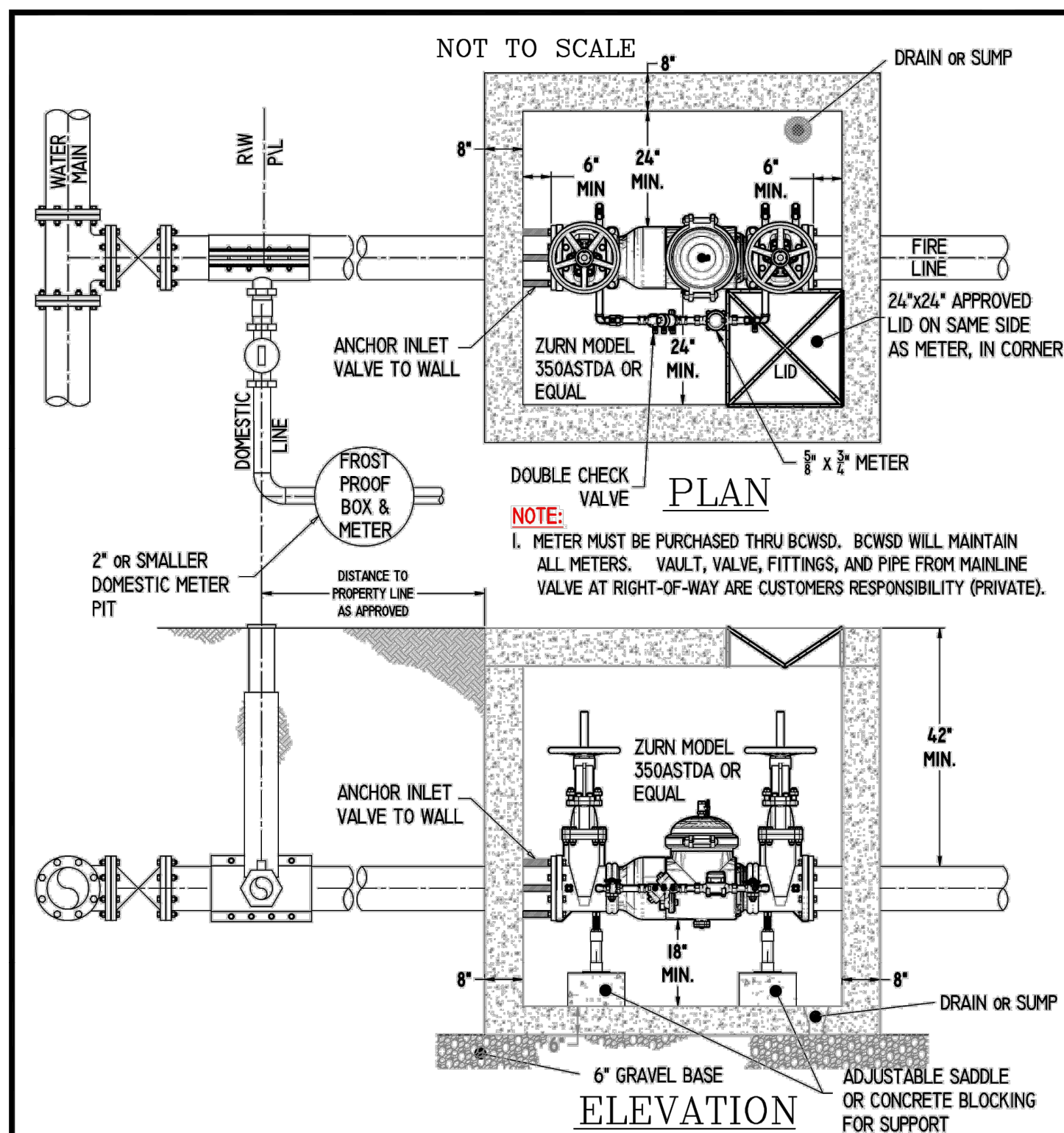


STANDARD DETAILS
UTILITIES RELATIONSHIP DETAIL

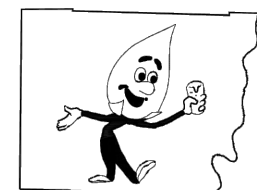
DWG. NO.
9

1/29/18

16



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

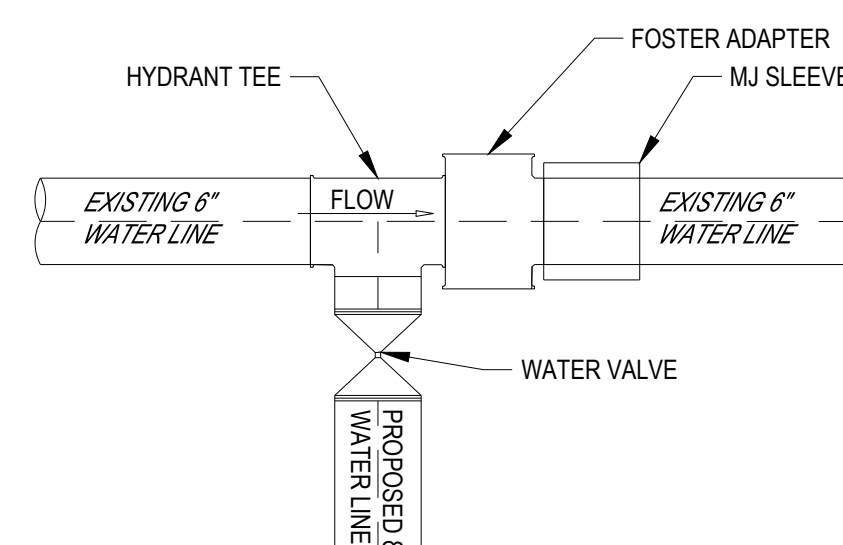


STANDARD DETAILS
OUTSIDE METER VAULT FOR SEPARATE FIRELINE & SEPARATE DOMESTIC SERVICE LINE WITH 2" OR SMALLER DOMESTIC METER

DWG. NO.
3D

2/15/24

8D



TIE-IN TO EXISTING WATER LINE
ON RECO ROAD

NOTE: FITTING TO BE A MINIMUM OF 5 FEET FROM ANY BELL OR OTHER FITTINGS OR TAPS



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



W W W . C E S O . I N C . C O M

175 Montrose West Ave., Suite 400
Akron, OH 44321
Phone: 330.665.0960 Fax: 888.208.4826



LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43718

Revisions / Submissions

ID	Description	Date
1	REV 1	01/03/2024
3	ADDENDUM 3	02/09/2024
4	ADDENDUM 4	02/23/2024

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Project Number: 759267
Scale: N/A
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
CONSTRUCTION
DETAILS

C9.4



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



CESO
WWW.CESOHINC.COM

175 Montrose West Ave., Suite 400
Alvord, OH 44321
Phone: 330.665.0960 Fax: 888.208.4826



12/18/2023

LOVE'S TRAVEL STOP

ST. CLAIRSVILLE, OH

66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

NYLOPLAST 10" INLINE DRAIN: 2710AG __ X

GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PEDESTRIAN (STANDARD)	LIGHT DUTY	1099C02	7001-110-199
SOLID COVER	LIGHT DUTY	1099C02	7001-110-199
BRONZE	NA	1099C08	7001-110-200
DOMED	NA	1099C02	7001-110-201

DRAWN BY EBC	MATERIAL		3150 VERONA AVE BURLINGTON, GA 30518 PHN (770) 932-2443 FAX (770) 932-2480 www.nyloplast-us.com	
DATE 11-15-06				
REVISED BY NHK	PROJECT NO. NAME	15 IN INLINE DRAIN QUICK SPEC INSTALLATION DETAIL		
DATE 03-15-16				
DWG SIZE A	SCALE 1:16	SHEET 1 OF 1	DWG NO. 7003-110-034	REV F

NYLOPLAST 10" INLINE DRAIN: 2710AG __ X

GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
STANDARD	LIGHT DUTY	1099C02	7001-110-199
SOLID COVER	LIGHT DUTY	1099C02	7001-110-199
BRONZE	NA	1099C08	7001-110-200
DOMED	NA	1099C02	7001-110-201

DRAWN BY CJA	MATERIAL		3150 VERONA AVE BURLINGTON, GA 30518 PHN (770) 932-2443 FAX (770) 932-2480 www.nyloplast-us.com	
DATE 06-25-09				
REVISED BY CCA	PROJECT NO. NAME	10 IN DESIGN DETAILS		
DATE 07-18-13				
DWG SIZE A	SCALE NTS	SHEET 1 OF 1	DWG NO. 7003-110-001	REV G

NYLOPLAST 15" INLINE DRAIN: 2715AG __ X

GRATE OPTIONS	LOAD RATING	PART #	DRAWING #
PEDESTRIAN	MEETS H-30	1599C01	7001-110-207
STANDARD	MEETS H-30	1599C01	7001-110-207
SOLID COVER	MEETS H-30	1599C01	7001-110-207
PEDESTRIAN (BRONZE)	NA	1599C08	7001-110-214
DOMED	NA	1599C01	7001-110-214
EMERGENCY GRATE	LIGHT DUTY	169101	7001-110-214

DRAWN BY EBC	MATERIAL		3150 VERONA AVE BURLINGTON, GA 30518 PHN (770) 932-2443 FAX (770) 932-2480 www.nyloplast-us.com	
DATE 04-03-06				
REVISED BY NHK	PROJECT NO. NAME	15 IN INLINE DRAIN QUICK SPEC INSTALLATION DETAIL		
DATE 03-15-16				
DWG SIZE A	SCALE 1:20	SHEET 1 OF 1	DWG NO. 7003-110-026	REV E

Revisions / Submissions		
ID	Description	Date

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Project Number: 759267
Scale: N/A
Drawn By: FAR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
CONSTRUCTION DETAILS

C9.5

LOVES TRAVEL STOP

ST. CLAIRSVILLE, OH

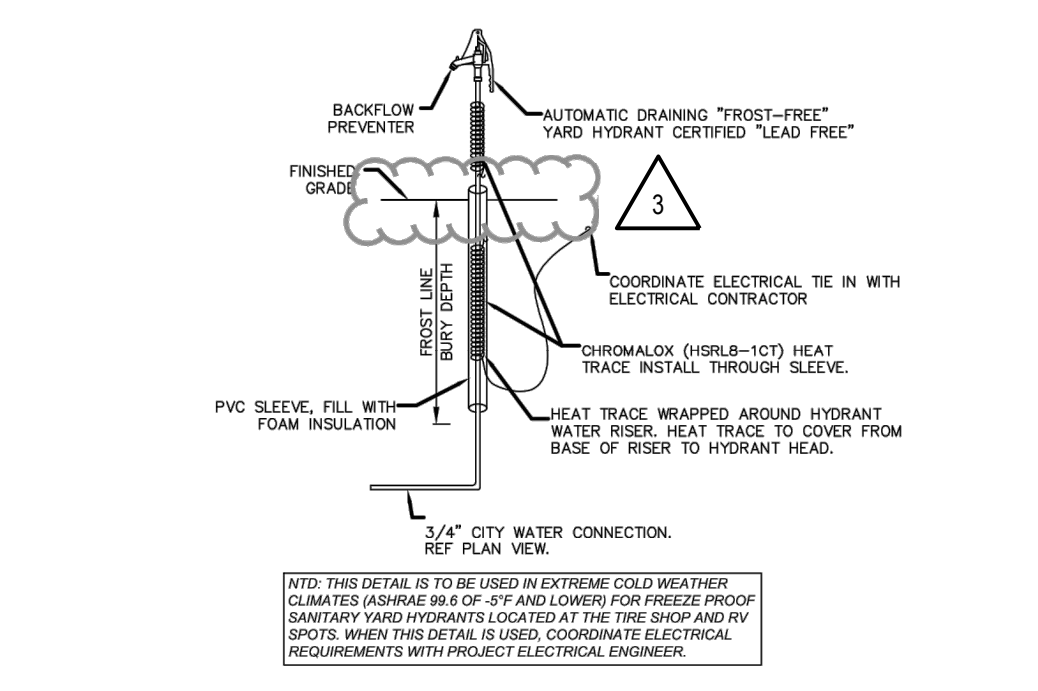
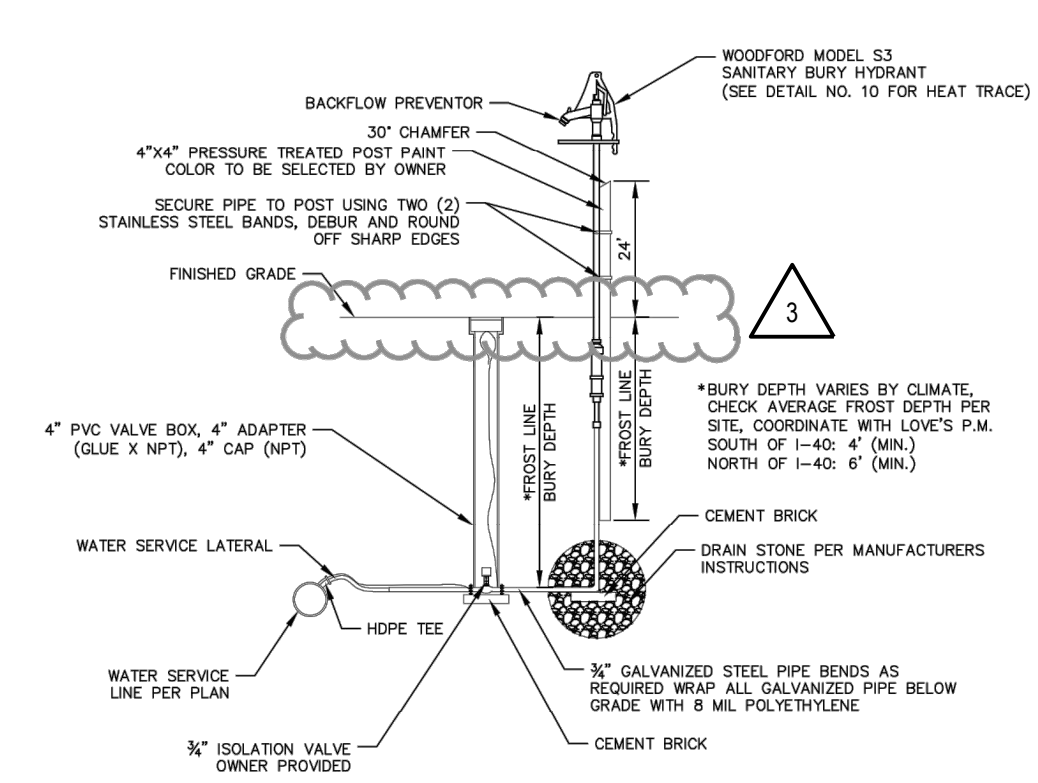
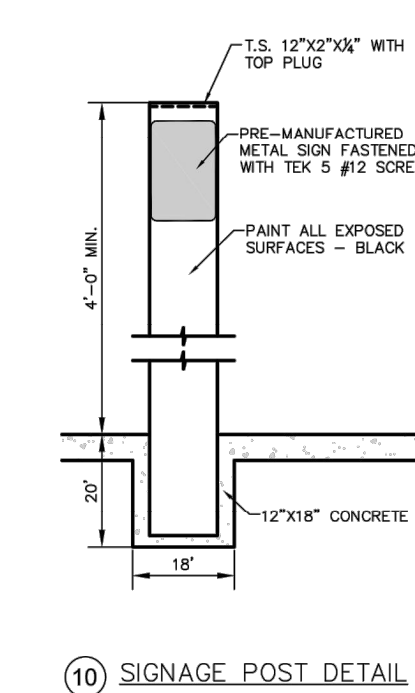
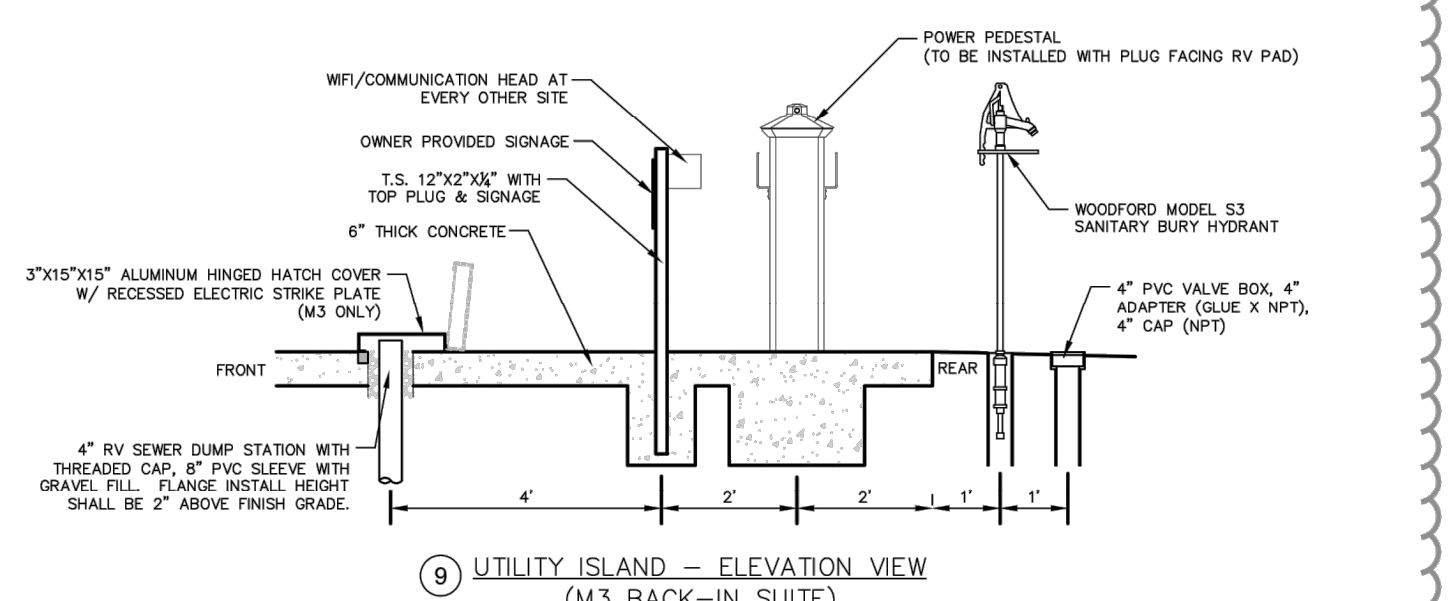
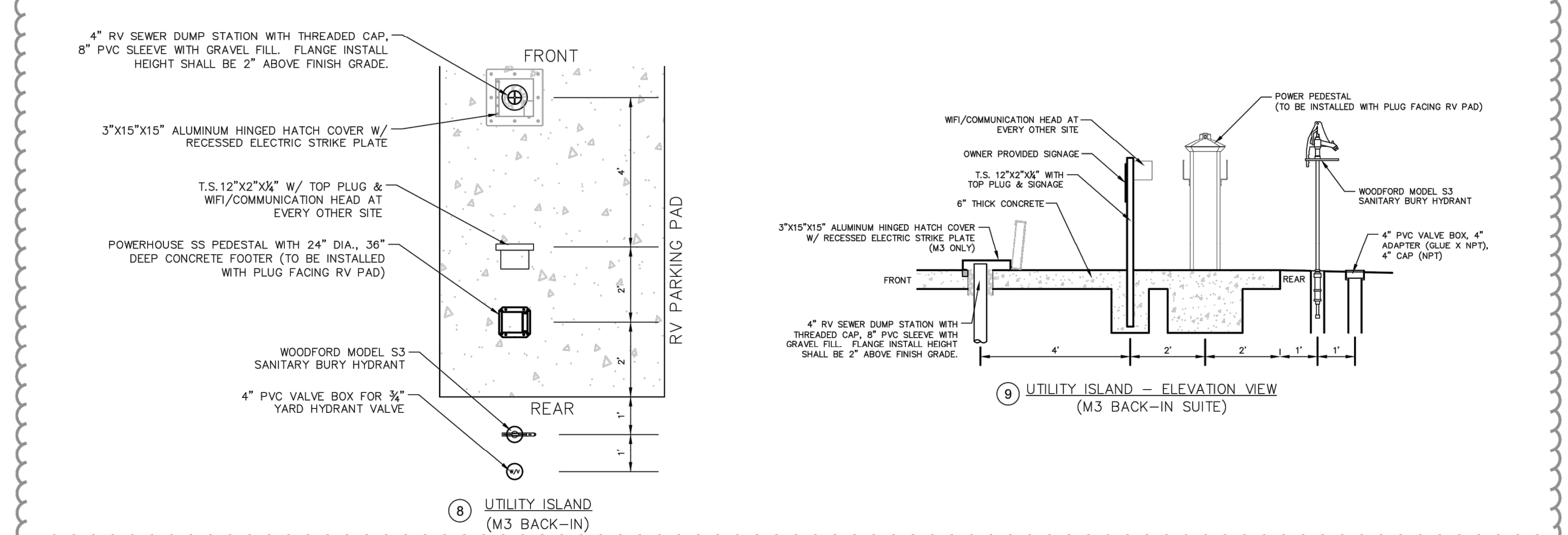
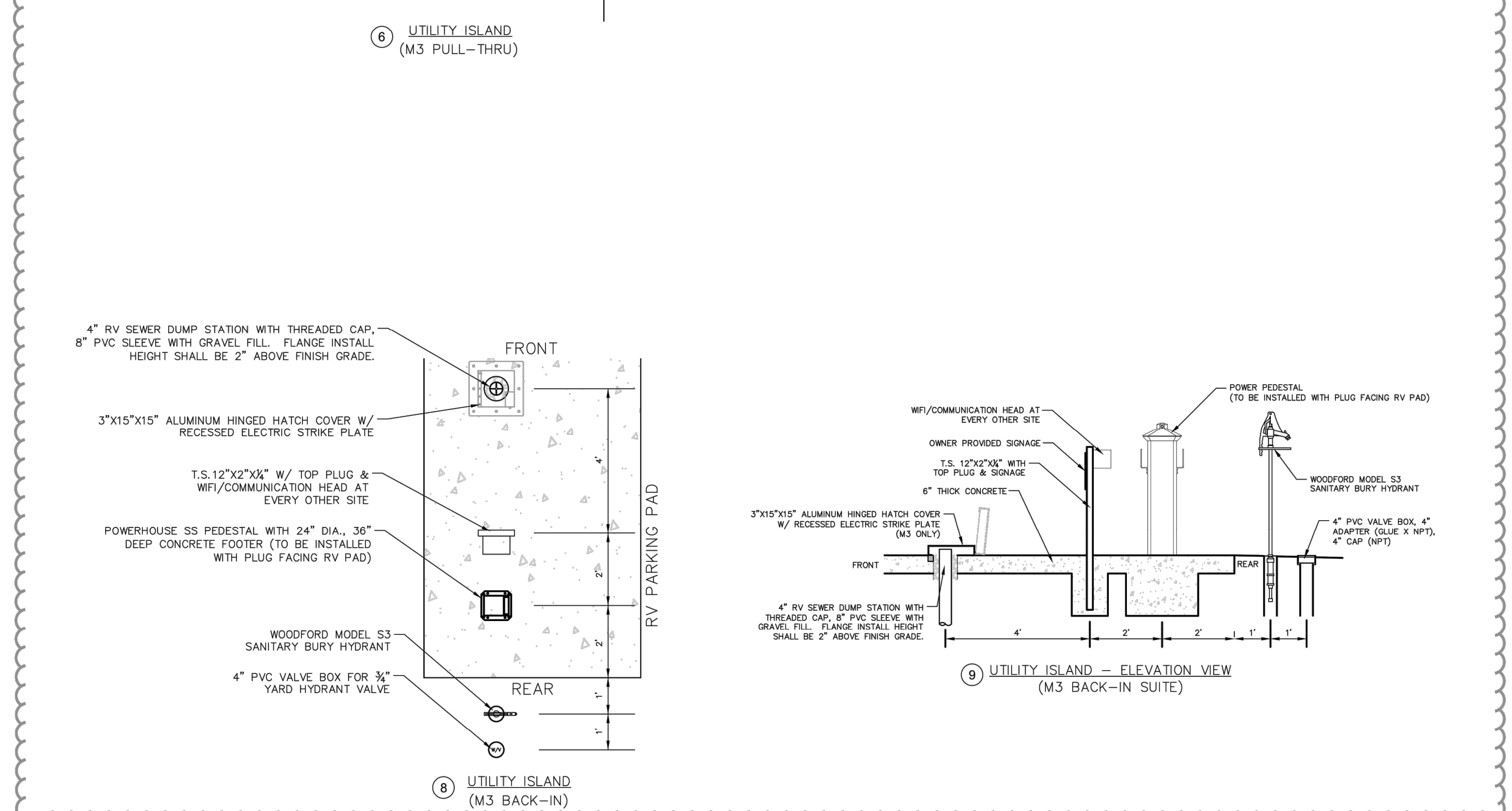
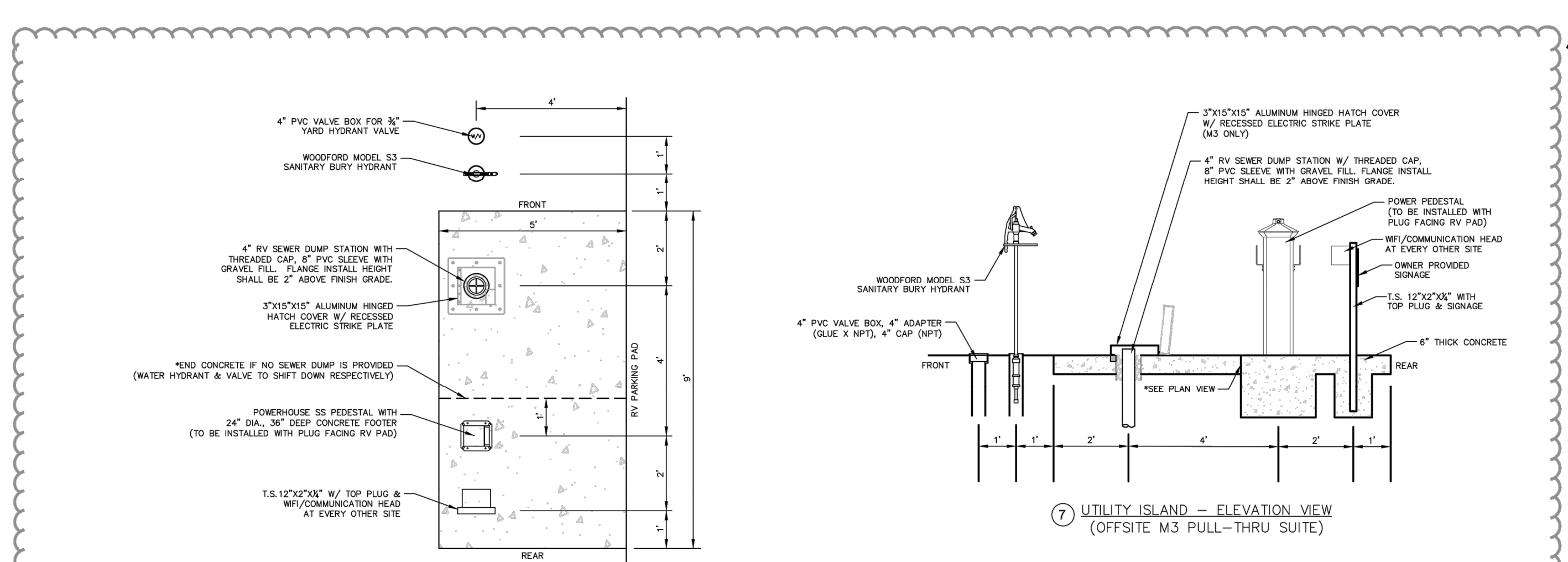
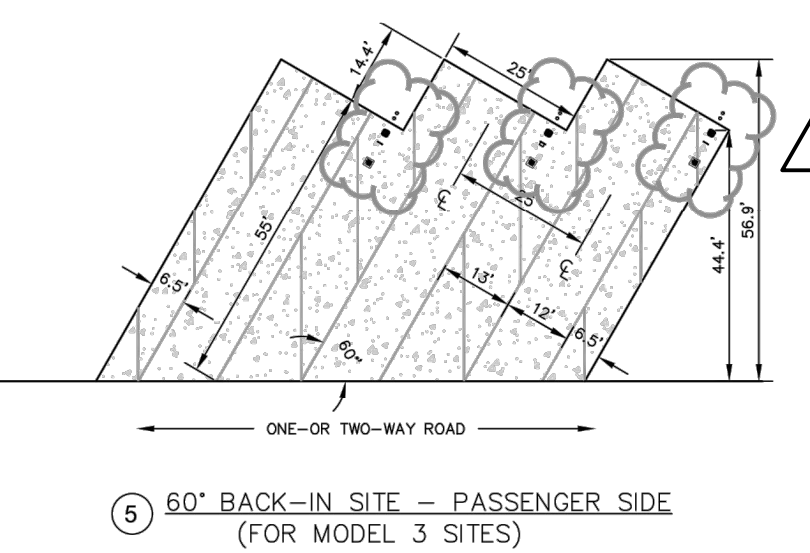
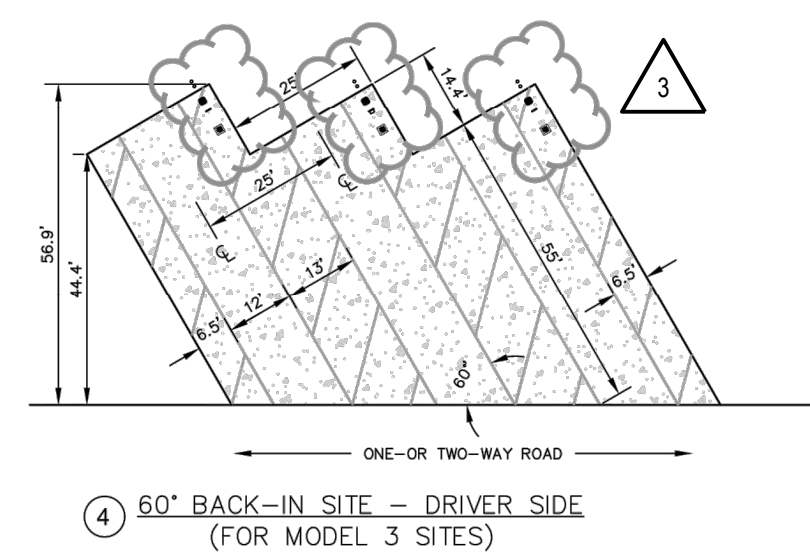
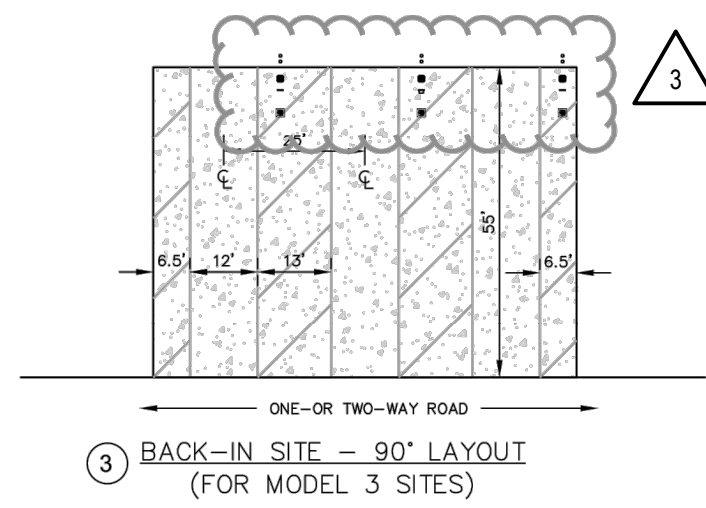
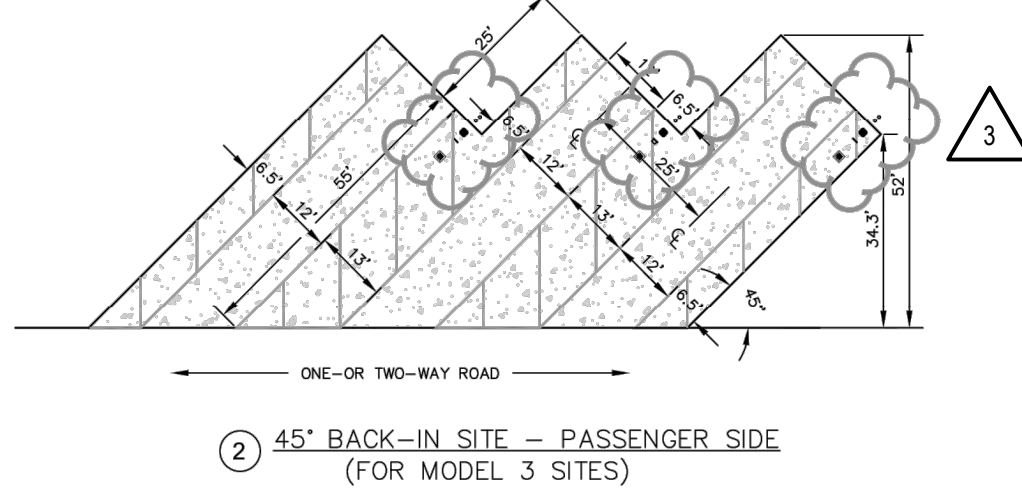
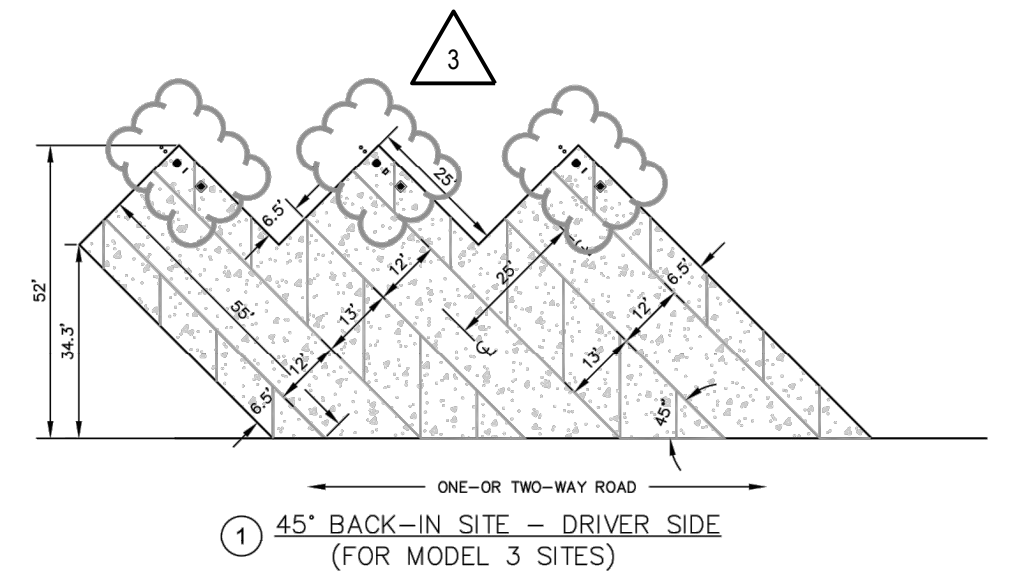
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions		
ID	Description	Date
3	ADDENDUM 3	02/09/2024

Project Number:	759267
Scale:	40
Drawn By:	CDR
Checked By:	JTK
Date:	12/19/2023
Issue:	OUT TO BID

Drawing Title:
CONSTRUCTION DETAILS

C9.6



NOTE: THIS DETAIL IS TO BE USED IN EXTREME COLD WEATHER CLIMATES (ASHRAE 9B & 9C) AND LOWER FROST PROOF SANITARY BURY HYDRANTS LOCATED AT THE FIRE SHOP AND RV SPOTS. WHEN THIS DETAIL IS USED, COORDINATE ELECTRICAL REQUIREMENTS WITH PROJECT ELECTRICAL ENGINEER.

FOR PERMITTING ONLY

Chipola Engineering Group, Inc.

CIVIL ENGINEERING, DESIGN, PLANNING, ENVIRONMENTAL PERMITTING, & CONSTRUCTION SERVICES
4420 JACKSON STREET, MARIANNA, FL 32448
PHONE (850) 372-4045
FIRM LICENSE NO.: XXXXXX

DRAWN BY:	JDK	DATE:	1/29/24
CHECKED BY:	HDW	DATE:	1/29/24
DESIGNED BY:	JDK	RV-PROTO	
SCALE:	N.T.S.	PLOT DATE:	1/29/24

REVISIONS		
DESCRIPTION	INT.	DATE

CLIENT : **RVSTOP Loves**

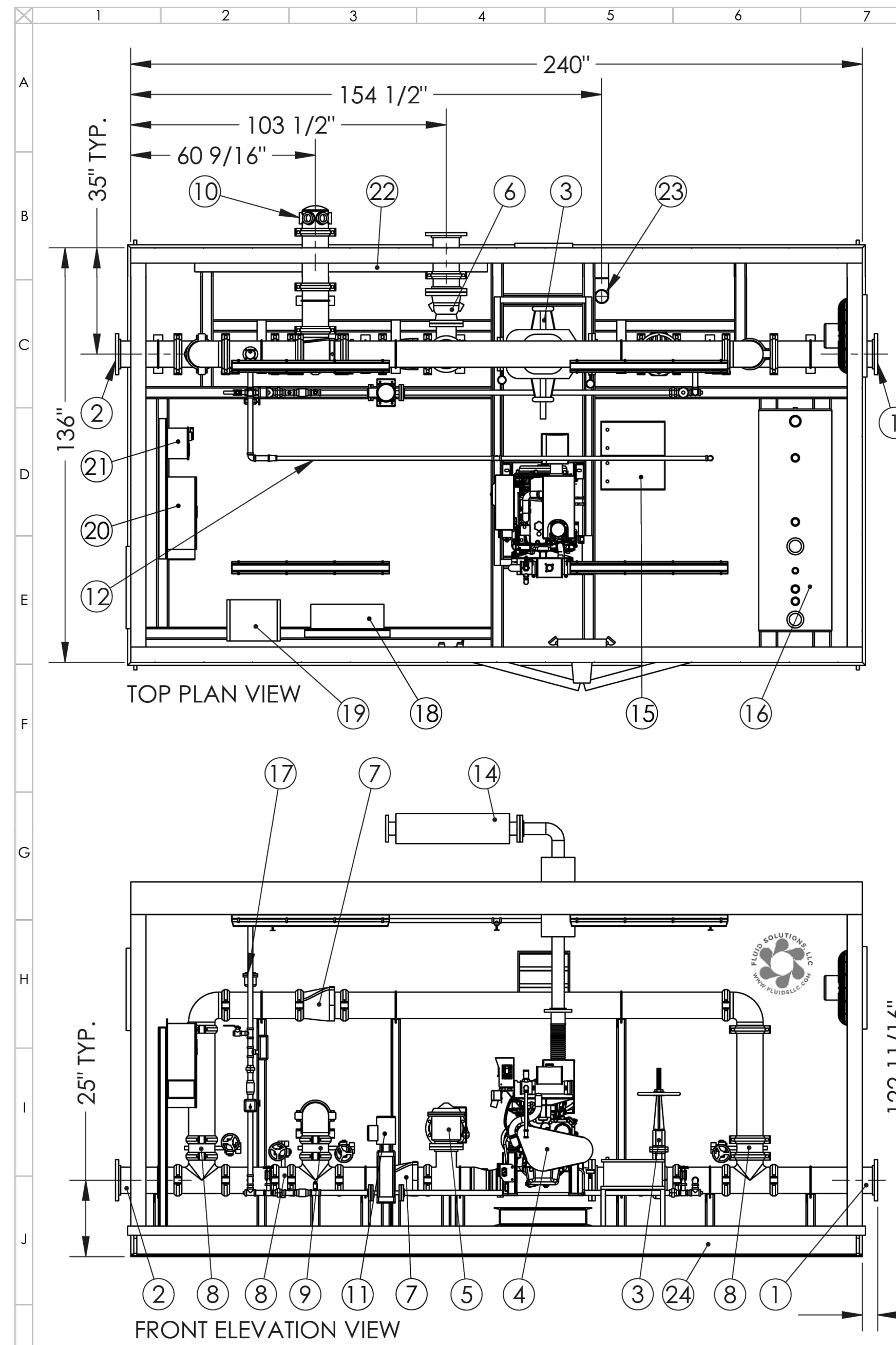
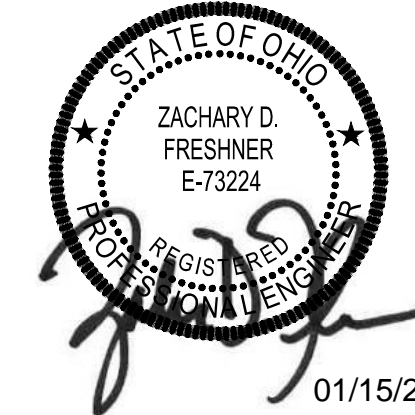
PROJECT : **JOB LOCATION, JOB STATE**

SHEET TITLE : **CONSTRUCTION DETAILS (M3 SITES)**

PROJ. NO. :	RV-PROTO
SHEET NO. :	C9.0



FORTY-EIGHT (48) HOURS BEFORE DIGGING IS TO COMMENCE, THE CONTRACTORS SHALL NOTIFY THE FOLLOWING AGENCIES: OHIO UTILITIES PROTECTION SERVICE AT 811 OR 800-362-2764 AND ALL OTHER AGENCIES WHICH MIGHT HAVE UNDERGROUND UTILITIES INVOLVING THIS PROJECT AND ARE NONMEMBERS OF STATE UTILITIES PROTECTION SERVICE



- NOTES:**
- 1) ANCHOR BOLT HOLES AVAILABLE ONLY UPON CUSTOMER REQUEST.
 - 2) ONLY MAIN STRUCTURAL SUPPORTS ARE SHOWN ON DRAWING. ADDITIONAL CROSS MEMBERS ARE REQUIRED TO SUPPORT PIPING, PUMP CONTROLLER, OR ANY OTHER APPLICABLE COMPONENTS. IF THERE IS A DRAIN OR ANY OTHER OBSTRUCTION IN THE FLOOR THAT SHOULD BE CONSIDERED, PLEASE LOCATE ON THIS DRAWING.
 - 3) THIS SKID IS DESIGNED TO BE FILLED WITH CONCRETE AT INSTALLATION.
 - 4) TAMPER SWITCHES ARE PROVIDED ON ALL GATE VALVES AND BUTTERFLY VALVES.
 - 5) BUILDING HVAC, EMERGENCY LIGHT, LIGHTS, JOCKEY PUMP CONTROL PANEL, JOCKEY PUMP, AND OTHER ITEMS ARE SHOWN IN THEIR GENERAL LOCATIONS AND MAY VARY FROM THE LOCATION SHOWN ON THE DRAWING.
 - 6) ELECTRICAL SERVICE TO BE LANDED IN DISTRIBUTION PANEL BY OTHERS.
 - 7) SPRINKLER LINE LAYOUT TO BE FINALIZED BY AN NFPA 13 CERTIFIED VENDOR AT TIME OF PROJECT RELEASE.

ITEM NO.	DESCRIPTION	QTY.
1	8" SYSTEM SUCTION CONNECTION, 150# FF	1
2	6"/8" SYSTEM DISCHARGE CONNECTION, 150# FF	1
3	8" OS&Y GATE VALVE ANSI 150#	1
4	1000/1250 GPM DIESEL DRIVEN PUMP	1
5	MAIN RELIEF VALVE 6"	1
6	WASTECON 6" X 8"	1
7	8" GROOVED CHECK VALVE	2
8	8" GROOVED BUTTERFLY VALVE (N.O.)	3
9	8" GROOVED BUTTERFLY VALVE (N.C.)	1
10	8"X6 GROOVED HOSE HEADER (SHIP LOOSE)	1
11	JOCKEY PUMP	1
12	SPRINKLER SYSTEM	1
13	1/2" DRIP VALVE	1
14	4" 150# FLG RESIDENTIAL MUFFLER (SHIP LOOSE)	1
15	12V BATTERY WITH RACK (SHIP LOOSE)	2
16	DOUBLE WALL FUEL TANK 140 GALLON	1
17	1/2" AIR RELEASE VALVE	1
18	SINGLE POINT POWER CONNECTION	1
19	MINI POWER ZONE	1
20	MAIN PUMP CONTROLLER	1
21	JOCKEY PUMP CONTROLLER	1
22	BASEBOARD HEATER	1
23	4" SYSTEM DRAIN	1
24	10" SYSTEM SKID	1

APPROVED
SIGN: _____
DATE: _____

APPROXIMATE DRY SYSTEM WEIGHT: 24,000 LBS

B	INCLUDED SUCTION AND DISCHARGE LABELS IN BOM	JRD	8/17/2021
A	UPDATED TO STANDARD SIZE	JRD	8/5/2021
REV	DESCRIPTION	BY	DATE
ALL DIMENSIONS +/- TOLERANCE OF 1/2"			

LOVES 1000 / 1250 GPM CITY FED SYSTEM HOUSE PACKAGE 2

DRAWN BY: JRD | ENG. BY: JRD | CHECKED BY: MDT
DATE: 7/28/2021

FLUID SOLUTIONS, LLC
a THG International Company
101 Lyon Lane Birmingham, AL 35211
205 705 1427 ph | www.fluidslc.com

SCALE: 1:42 DWG: A1-1

LOVES TRAVEL STOP
ST. CLAIRSVILLE, OH
66320 BELMONT-MORRISTOWN ROAD, UNION TOWNSHIP, OH 43178

Revisions / Submissions

ID	Description	Date
1	REV 1	01/03/2024

© 2023 CESO, INC.
Project Number: 759267
Scale: 1" = 40'
Drawn By: CDR
Checked By: JTK
Date: 12/19/2023
Issue: OUT TO BID

Drawing Title:
CONSTRUCTION DETAILS

C9.7

ROADWAY IMPROVEMENTS LOVE'S TRAVEL STOPS BEL-149-23.44

UNION TOWNSHIP BELMONT COUNTY, OH

INDEX OF SHEETS

TITLE SHEET	R1
SCHEMATIC PLAN	R2
GENERAL NOTES	R3
TYPICAL SECTIONS	R4-R6
MAINTENANCE OF TRAFFIC	R7-R11
SITE PLAN	R12
PLAN AND PROFILE	R13-R16
CROSS SECTIONS	R17-R25
INTERSECTION DETAILS	R26-R28
PAVEMENT MARKING AND SIGNAGE PLAN	R29-R30
CONSTRUCTION DETAILS	R31
TRAFFIC SIGNAL PLAN	T1-T14

PROJECT DESCRIPTION

CONSTRUCTION TO WIDEN EAST AND WEST SIDE OF S.R. 149 TO PROVIDE 175' NB LEFT TURN LANE INTO LOVE'S DRIVEWAY, 175' SB RIGHT-TURN LANE INTO LOVE'S DRIVEWAY, 164' TWLTL BETWEEN SOUTH PILOT DRIVEWAY & NORTH PILOT DRIVEWAY, 125' NB LEFT TURN LANE INTO RECO DRIVE, 200' SB RIGHT TURN LANE INTO RECO DRIVE AND ASSOCIATED LANE RECONFIGURATION TO ACCOMMODATE AFOREMENTIONED IMPROVEMENTS. CONSTRUCTION TO WIDEN RECO DRIVE AND INSTALLATION OF TRAFFIC SIGNAL AT S.R. 149 & RECO DRIVE INTERSECTION. DITCH AND SLOPE GRADING AND INSTALLATION OF NECESSARY TRAFFIC CONTROL DEVICES. STORMWATER STRUCTURE AND CONDUIT MODIFICATIONS BETWEEN SOUTH MARATHON DRIVEWAY AND NORTH MARATHON DRIVEWAY.

EARTH DISTURBED AREAS AND TOTAL AREA OF PROJECT

TOTAL AREA OF PROJECT: 1.95 ACRES.
PROJECT EARTH DISTURBING ACTIVITIES AREA: 1.65 ACRES
CONTRACTOR EARTH DISTURBING ACTIVITIES AREA: 0.30 ACRES
NOI EARTH DISTURBING ACTIVITIES AREA: 1.95 ACRES

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY AND THAT PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE AS SET FORTH ON THE PLANS AND ESTIMATES.

APPROVED _____

DATE _____ DISTRICT DEPUTY DIRECTOR

APPROVED _____

DATE _____ DIRECTOR, DEPARTMENT OF TRANSPORTATION

RAILROAD INVOLVEMENT
NONE

FEDERAL PROJECT NUMBER
NON-FEDERAL

PLAN PREPARED BY:



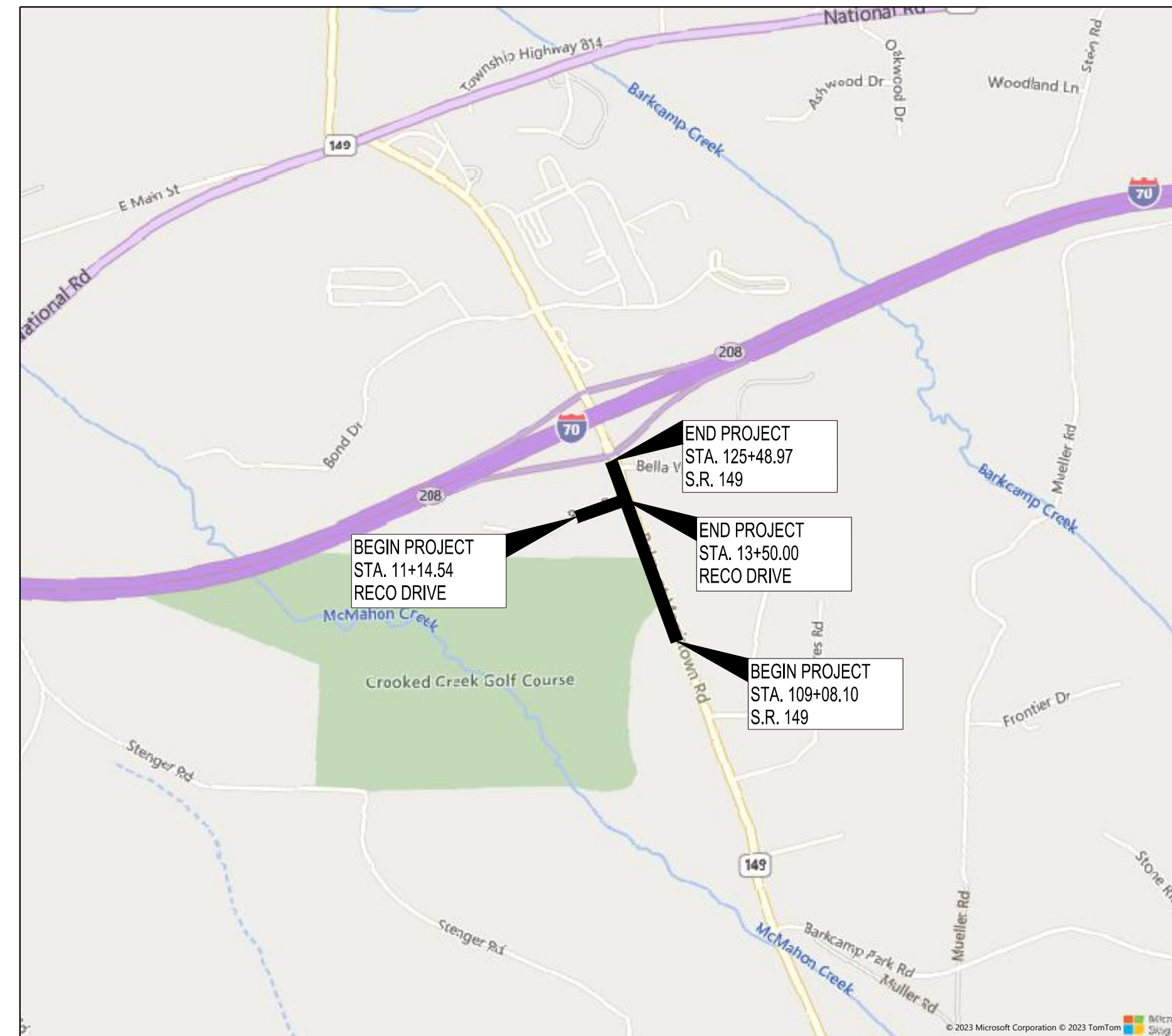
WWW.CESOINC.COM

3601 RIGBY ROAD, SUITE 300
MIAMISBURG, OHIO 45342
PH (937) 435-8584 | FAX (937) 435-3307

ENGINEER'S STAMP

SIGNED: _____

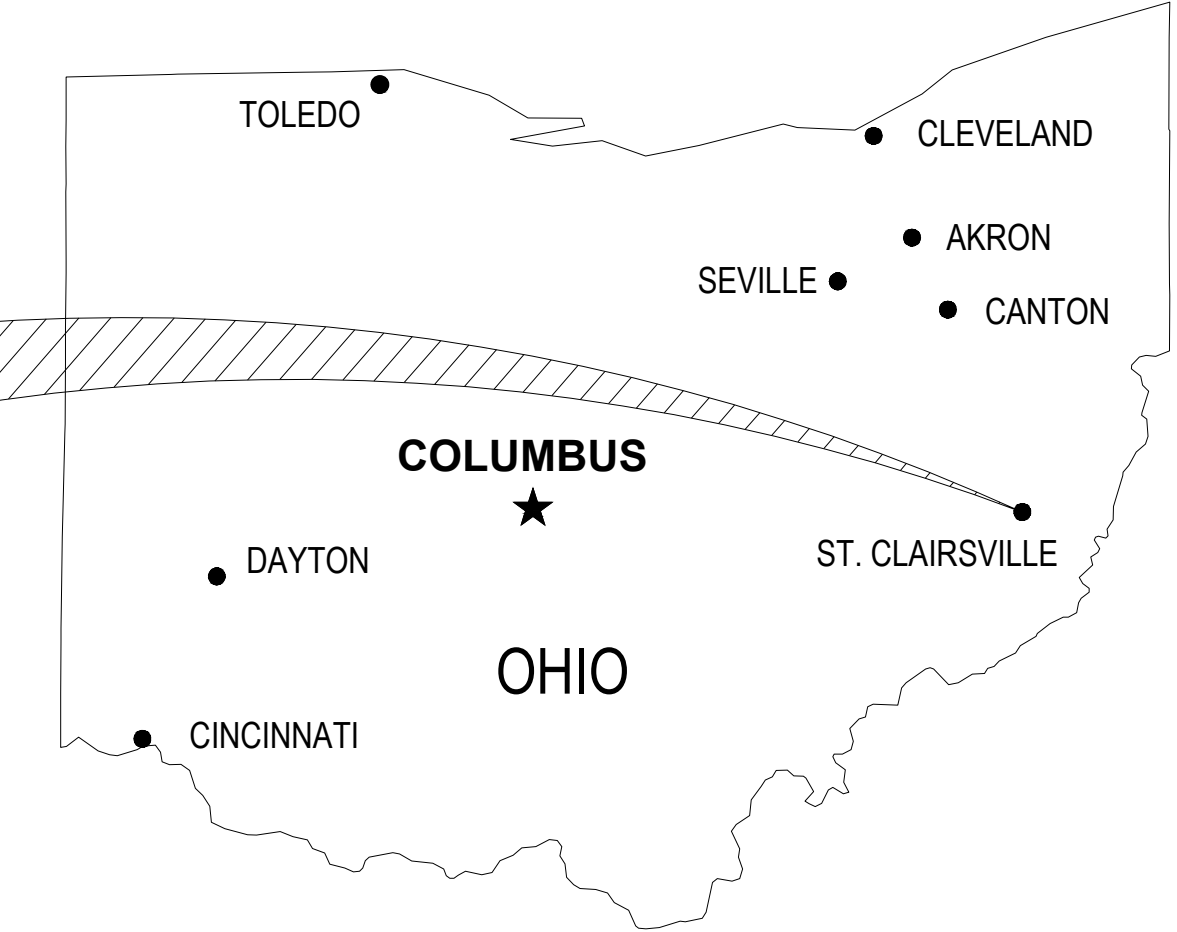
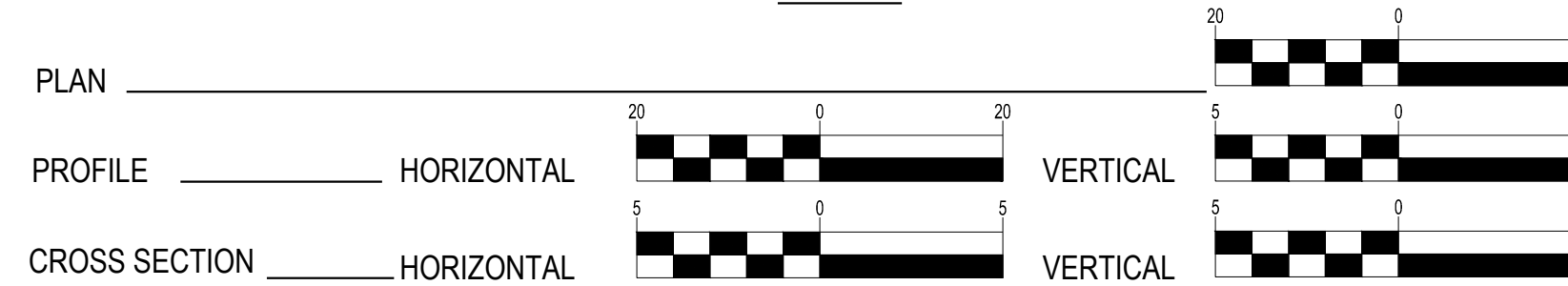
DATE: _____



LOCATION MAP
NOT TO SCALE

PROJECT LOCATED ALONG S.R. 149 - BELMONT MORRISTOWN ROAD, 1.29 MILES SOUTHEAST OF MORRISTOWN, OHIO AND SOUTH OF THE THE S.R. 149 AND I-70 INTERCHANGE. APPROXIMATELY 8.00 MILES WEST OF CLAIRSVILLE, OHIO

SCALES



STATE LOCATION MAP
NOT TO SCALE

SITE LOCATION

LAT = N 40° 03' 27"
LONG = W 81° 03' 09"

DESIGN DESIGNATION - STATE ROUTE 149 (BELMONT MORRISTOWN ROAD):

AADT (2024)	10,000
DESIGN YEAR AADT (2044)	14,500
DESIGN HOURLY VOLUME (2044)	1,300
DIRECTIONAL DISTRIBUTION	55%
TRUCKS (24 HOUR B&C)	21%
DESIGN SPEED	50 MPH
LEGAL SPEED	45 MPH
DESIGN FUNCTIONAL CLASSIFICATION	5 - RURAL MAJOR COLLECTOR
NHS PROJECT	NO

2023 SPECIFICATIONS

THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO DEPARTMENT OF TRANSPORTATION INCLUDING CHANGES AND SUPPLEMENTAL SPECIFICATIONS LISTED IN PLANS AND CHANGES LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

**PRELIMINARY
NOT FOR
CONSTRUCTION**

UNDERGROUND UTILITIES
Contact Two Working Days
Before You Dig

OHIO811, 8-1-1, or 1-800-362-2764
(Non-members must be called directly)

STANDARD CONSTRUCTION DRAWINGS										SUPPLEMENTAL SPECIFICATIONS		REVISIONS		
ODOT HW-2.2	7/20/2018	ODOT DM-1.1	7/17/2020	ODOT MT-95.31	7/19/2019	ODOT TC-41.41	7/19/2019	ODOT TC-81.22	7/21/2023	SS 800	1/19/2024	BY	DATE	DESCRIPTION
		ODOT DM-1.2	7/16/2021	ODOT MT-97.11	1/20/2017	ODOT TC-42.20	10/18/2013	ODOT TC-83.10	1/17/2020			PD	11/8/2023	ODOT COMMENTS #1
ODOT CB-3A	7/16/2021	ODOT DM-4.3	1/15/2016	ODOT MT-99.20	7/19/2019	ODOT TC-52.10	10/18/2013	ODOT TC-83.20	1/19/2024	SS 832	7/21/2023	PD	1/2/2024	ODOT COMMENTS #2
		ODOT DM-4.4	1/15/2016	ODOT MT-101.90	7/17/2020	ODOT TC-52.20	1/15/2021	ODOT TC-85.20	4/1/2023	SS 863	7/21/2023	PD	1/3/2024	REV 1
ODOT BP-3.1	1/21/2022			ODOT MT-120.00	1/19/2024	ODOT TC-61.30	7/19/2019			SS 809	1/19/2024	PD	2/9/2024	ADDENDUM #3
ODOT BP-4.1	7/19/2013	ODOT MGS-1.1	7/16/2021									PD	2/23/2024	ODOT COMMENTS #3
ODOT BP-5.1	7/15/2022	ODOT MGS-2.1	1/19/2018	ODOT TC-21.21	1/20/2023	ODOT TC-65.10	1/17/2014			SS 825	4/21/2023	PD	2/23/2024	ADDENDUM #4
		ODOT MGS-4.2	7/19/2013	ODOT TC-41.20	10/18/2013	ODOT TC-65.11	7/15/2022			SS 909	1/19/2024	PD	4/29/2024	ODOT COMMENTS #4
				ODOT TC-41.30	4/21/2023	ODOT TC-71.10	4/21/2023							

DATE
4-29-2024

ISSUE
AGENCY REVIEW

CALCULATED
P.I.D.
CHECKED
TDH

TITLE SHEET
S.R. 149

BEL-149-23.44

R1
R31

BENCHMARKS	
BM #A	"X" CHISELED IN THE SOUTH SIDE OF RIM OF SANITARY MANHOLE ON NORTH SIDE OF RECO DRIVE, EAST OF ADDRESS 41245 RECO DRIVE ELEV=1239.44'
BM #B	EAST BOLT ON HYDRANT ON SOUTH SIDE OF RECO DRIVE, ACROSS FROM ADDRESS 41371 RECO DRIVE ELEV=1207.05'
BM #C	SOUTHWEST BOLT ON HYDRANT ON EAST SIDE OF BELMONT MORRISTOWN ROAD AT ADDRESS 66377 BELMONT MORRISTOWN ROAD ELEV=1193.84'

BENCHMARK NOTES:
 BM "A" AND BM "B" LOCATED OUTSIDE OF PROJECT VIEWPORT LIMITS. REFER TO ORIGINAL SURVEY CONDUCTED BY CESO, INC. DATED APRIL 12, 2023 FOR MARKED LOCATION OF BENCHMARKS.

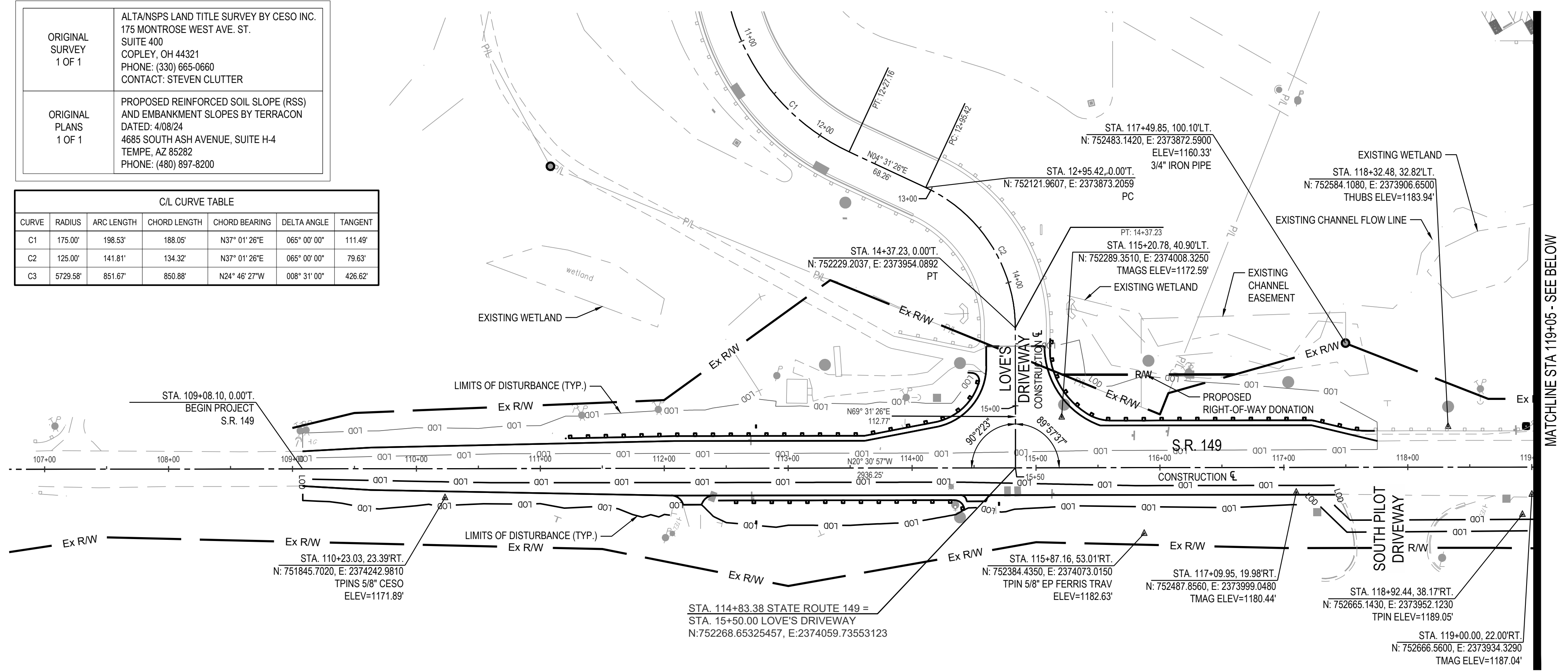
DATUM NOTES:
HORIZONTAL DATUM:
 OHIO STATE PLANE COORDINATE SYSTEM, OHIO SOUTH ZONE, NAD83 (2011) AS DETERMINED BY OPUS POST PROCESSED STATIC SESSION.
VERTICAL DATUM:
 NAVD 88 AS DETERMINED BY OPUS POST PROCESSED STATIC SESSIONS USING GEOID 18 (CONUS)
UTILITY DISCLAIMER:
 THE UTILITIES SHOWN HEREON HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION OUPS #A218001060 AND/OR EXISTING DRAWINGS. THE SURVEYOR MAKES NO GUARANTEE THAT THE UTILITIES LOCATED HERE ON COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UTILITIES LOCATED ARE IN THE EXACT LOCATION INDICATED ALTHOUGH HE DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.
 DATE OF FIELD SURVEY: AUGUST 9, 2022
 DATE OF SURVEY MAP: APRIL 12, 2023

LANDSCAPE NOTE:
 THERE ARE NO EXISTING LANDSCAPED AREAS WITHIN THE WORK LIMITS.

REFERENCE DRAWINGS:

ORIGINAL SURVEY 1 OF 1	ALTA/SPS LAND TITLE SURVEY BY CESO INC. 175 MONTROSE WEST AVE. ST. SUITE 400 COPLE, OH 44321 PHONE: (330) 665-0660 CONTACT: STEVEN CLUTTER
ORIGINAL PLANS 1 OF 1	PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES BY TERRACON DATED: 4/08/24 4685 SOUTH ASH AVENUE, SUITE H-4 TEMPE, AZ 85282 PHONE: (480) 897-8200

C/L CURVE TABLE						
CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE	TANGENT
C1	175.00'	198.53'	188.05'	N37° 01' 26"E	065° 00' 00"	111.49'
C2	125.00'	141.81'	134.32'	N37° 01' 26"E	065° 00' 00"	79.63'
C3	5729.58'	851.67'	850.88'	N24° 46' 27"W	008° 31' 00"	426.62'



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SPECIFICATIONS AND STANDARD CONSTRUCTION DRAWINGS

THE STATE OF OHIO DEPARTMENT OF TRANSPORTATION, 2023 CONSTRUCTION AND MATERIALS SPECIFICATIONS, AND THE NOTED STANDARD CONSTRUCTION DRAWINGS SHALL GOVERN THIS IMPROVEMENT. WHEREVER THE WORD "STATE" OCCURS, IT IS TO MEAN THE OHIO DEPARTMENT OF TRANSPORTATION (ODOT).

WHENEVER THE WORD "ENGINEER" OR "DEPARTMENT" IS USED HEREIN, IT SHALL BE HELD TO MEAN THE ODOT PROJECT ENGINEER, OR DULY AUTHORIZED AUTHORITY.

EXISTING SURVEY MONUMENTS

EXISTING SURVEY MONUMENTS, INCLUDING BUT NOT LIMITED TO PROPERTY CORNERS, CENTERLINE MONUMENTS, RIGHT-OF-WAY MONUMENTS, ETC., MAY POTENTIALLY BE DISTURBED OR DESTROYED BY CONSTRUCTION OF THESE ROADWAY IMPROVEMENTS. THE CONTRACTOR SHALL RETAIN A PROFESSIONAL SURVEYOR, LICENSED IN THE STATE OF OHIO, TO IDENTIFY MONUMENTS THAT MAY BE DISTURBED AND TO PROPERLY REPLACE DISTURBED MONUMENTS IN-KIND AFTER CONSTRUCTION IS COMPLETE.

PERMITS FEES AND NOTICES

THE CONTRACTOR SHALL OBTAIN, AT HIS EXPENSE, ANY AND ALL PERMITS AND INSPECTIONS REQUIRED FOR THE PROSECUTION OF THE WORK BY LOCAL LAWS, ORDINANCES, RULES AND REGULATIONS.

MATERIALS AND WORKMANSHIP

UNLESS OTHERWISE SPECIFIED, ALL MATERIALS SHALL BE NEW AND BOTH WORKMANSHIP AND MATERIALS SHALL BE OF THE FIRST QUALITY, PROPER AND SUFFICIENT FOR THE PURPOSE CONTEMPLATED. THE CONTRACTOR SHALL FURNISH, IF SO REQUIRED, SATISFACTORY EVIDENCE AS TO THE KIND AND QUALITY OF MATERIALS AND WORKMANSHIP. ALL ITEMS OF EQUIPMENT AND/OR MATERIALS PROPOSED FOR SUBSTITUTION MUST BE APPROVED BY THE ENGINEER IN WRITING AND SHALL BE EQUAL OR BE SUPERIOR TO THE ITEMS SPECIFIED IN THE CONTRACT DOCUMENTS. IF SAID SUBSTITUTION PROPOSED BY THE CONTRACTOR FOR A SPECIFIED ITEM REQUIRED ENGINEERING REVISIONS, THE EXPENSE OF SUCH REVISIONS SHALL BE PAID FOR BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.

EXISTING FACILITIES

IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PERFORM HIS WORK IN SUCH A MANNER AS NOT TO DAMAGE OR DESTROY ANY EXISTING FACILITY. IF ANY SUCH DAMAGE DOES OCCUR DUE TO THE CONTRACTOR'S OPERATIONS, HE SHALL REPLACE THE DAMAGED PORTION AT HIS EXPENSE AND TO THE SATISFACTION OF THE ENGINEER.

EXAMINATION OF THE SITE

THE CONTRACTOR SHALL CAREFULLY EXAMINE THE SITE OF THE PROPOSED WORK, THE PLANS AND SPECIFICATIONS. THE SUBMISSION OF A BID SHALL BE CONSIDERED EVIDENCE THAT THE CONTRACTOR HAS MADE SUCH EXAMINATION AND IS SATISFIED AS TO THE CONDITIONS TO BE ENCOUNTERED IN PERFORMING THE WORK IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS.

WORK LIMITS

THE WORK LIMITS SHOWN ON THESE PLANS ARE FOR PHYSICAL CONSTRUCTION ONLY. THE INSTALLATION AND OPERATION OF ALL TEMPORARY TRAFFIC CONTROL AND TEMPORARY TRAFFIC CONTROL DEVICES REQUIRED BY THESE PLANS SHALL BE PROVIDED BY THE CONTRACTOR WHETHER INSIDE OR OUTSIDE THESE WORK LIMITS.

RELOCATION OF UTILITIES

ALL UTILITIES WHICH ARE SHOWN OR LOCATED DURING THE COURSE OF CONSTRUCTION THAT ARE FOUND TO BE IN CONFLICT WITH THESE PLANS ARE TO BE RELOCATED OR ADJUSTED BY THE OWNER OF THE UTILITY.

UNDERGROUND UTILITIES

THE LOCATIONS OF THE UNDERGROUND UTILITIES SHOWN ON THE PLAN AREAS OBTAINED FROM THE OWNERS OF THE UTILITIES. THE SURVEYOR MAKES NO GUARANTEE THAT THE UTILITIES LOCATED HEREON COMPROMISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UTILITIES LOCATED ARE IN THE EXACT LOCATION INDICATED ALTHOUGH HE/SHE DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.

SEEDING AND MULCHING

THE FOLLOWING QUANTITIES ARE PROVIDED TO PROMOTE GROWTH AND CARE OF PERMANENT SEEDING AREAS:

ITEM 659, TOPSOIL	661 CU.YD
ITEM 659, SEEDING AND MULCHING	3,965 SQ. YD.
ITEM 659, COMMERCIAL FERTILIZER	0.54 TON
ITEM 659, LIME	0.82 ACRES
ITEM 659, WATER	10.7 M.GAL

APPLY SEEDING AND MULCHING TO ALL AREAS OF EXPOSED SOIL BETWEEN THE RIGHT-OF-WAY LINES AND WITHIN THE CONSTRUCTION LIMITS FOR AREAS OUTSIDE THE RIGHT-OF-WAY LINES COVERED BY WORK AGREEMENT OR SLOPE EASEMENT. QUANTITY CALCULATIONS FOR SEEDING AND MULCHING ARE BASED ON THESE LIMITS.

ITEM 659 SEEDING AND MULCHING

SEEDING AND MULCHING SHALL BE APPLIED TO ALL AREAS OF EXPOSED SOIL BETWEEN THE RIGHT-OF-WAY LINES, AND WITHIN THE CONSTRUCTION LIMITS FOR AREAS OUTSIDE THE RIGHT-OF-WAY LINES COVERED BY WORK AGREEMENT OR SLOPE EASEMENT. QUANTITY CALCULATIONS FOR ITEM 659, SEEDING AND MULCHING, ARE BASED ON THESE LIMITS.

WEATHER CONDITIONS

NO CONSTRUCTION SHALL BE DONE DURING STORMY, FREEZING OR INCLEMENT WEATHER UNLESS PERMISSION IS GIVEN BY THE ENGINEER WHENEVER WORK PROCEEDS DURING SUCH WEATHER, THE CONTRACTOR SHALL PROVIDE APPROVED FACILITIES, INCLUDING HEAT (IF REQUIRED), FOR THE PROTECTION OF ALL MATERIALS AND FINISHED WORK.

AGENCY NOTIFICATIONS

CONTRACTOR TO NOTIFY LOCAL EMERGENCY SERVICE PROVIDERS, UNITED STATES POSTAL SERVICE, LOCAL SCHOOL DISTRICTS PRIOR TO START OF CONSTRUCTION.

REVIEW OF DRAINAGE FACILITIES

PRIOR TO THE START OF WORK AND AGAIN BEFORE FINAL ACCEPTANCE, PERFORM AN INSPECTION WITH REPRESENTATIVES OF THE DEPARTMENT, CONTRACTOR AND LOCALS OF ALL EXISTING DRAINAGE FACILITIES THAT ARE TO REMAIN IN SERVICE WHICH MAY BE AFFECTED BY THE WORK. THE CONDITION OF THE EXISTING CONDUITS AND THEIR APPURTENANCES IS DETERMINED FROM FIELD OBSERVATIONS. RECORDS OF THE INSPECTION ARE MAINTAINED BY THE DEPARTMENT. CONFIRM ALL EXISTING SEWERS INSPECTED INITIALLY BY THE ABOVE-MENTIONED PARTIES ARE MAINTAINED AND LEFT IN A CONDITION COMPARABLE TO THAT DETERMINED BY THE ORIGINAL INSPECTION. THE CONTRACTOR IS RESPONSIBLE TO CORRECT ANY CHANGE IN THE CONDITION RESULTING FROM THEIR OPERATIONS AS DIRECTED AND APPROVED BY THE ENGINEER. PAYMENT FOR ALL OPERATIONS DESCRIBED ABOVE IS INCLUDED IN THE CONTRACT PRICE FOR THE PERTINENT 611 CONDUIT ITEMS.

ROUNDING

THE ROUNDING AT SLOPE BREAKPOINTS SHOWN ON THE TYPICAL SECTIONS APPLIES TO ALL CROSS SECTIONS, EVEN THOUGH OTHERWISE SHOWN.

COOPERATION WITH UTILITY COMPANIES

WHILE THE WORK OF THIS CONTRACT IS BEING PERFORMED, THE UTILITY COMPANIES MAY BE WORKING IN THE AREA ADJUSTING AND RESETTING EXISTING FACILITIES. THE CONTRACTOR SHALL FULLY COOPERATE WITH UTILITY COMPANIES SO THAT THE ENTIRE WORK IS COMPLETED IN A MANNER CONSISTENT WITH GOOD CONSTRUCTION PRACTICES. THE CONTRACTOR, UTILITIES, AND ENGINEER SHALL DISCUSS THE NECESSARY CONSTRUCTION SCHEDULES TO COMPLETE THE PROJECT.

CROSSINGS TO EXISTING PIPES AND UTILITIES

WHERE PLANS PROVIDE FOR A PROPOSED CONDUIT TO CROSS OVER OR UNDER AN EXISTING SEWER OR UNDERGROUND UTILITY, THE CONTRACTOR SHALL LOCATE THE EXISTING PIPES OR UTILITIES BOTH AS TO LINE AND GRADE BEFORE STARTING THE PROPOSED CONDUIT.

UTILITY COORDINATION

THE FOLLOWING IS A LIST OF THE KNOWN EXISTING UTILITIES IN THE PROJECT AREA ALONG WITH THE RESPECTIVE OWNERS. THE LIST IS INCLUDED FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING ALL UTILITY OWNERS IN THE PROJECT AREA PRIOR TO START OF CONSTRUCTION WHETHER LISTED ON THIS PAGE OR NOT.

SANITARY SEWER
BELMONT COUNTY
WATER & SEWER DISTRICT
67711 OAK VIEW DRIVE
ST. CLAIRSVILLE, OH 43950
PH: (740) 695-3144
CONTACT: JEANNE KINNEY

FIRE
LAFFERTY VOLUNTEER
FIRE DEPARTMENT
70191 IRWIN ROAD
LAFFERTY, OH 43951
PH: (740) 310-0901
CONTACT: DUSTIN HUDAK

WATER
BELMONT COUNTY
WATER & SEWER DISTRICT
67711 OAK VIEW DRIVE
ST. CLAIRSVILLE, OH 43950
PH: (740) 695-3144
CONTACT: JEANNE KINNEY

OWNER/DEVELOPER
LOVE'S TRAVEL STOPS & COUNTRY STORES, INC.
10601 N. PENNSYLVANIA AVE.
OKLAHOMA CITY, OK 73120
CONTACT: RICK SHUFFIELD
PH: (405) 302-6646
EMAIL: RICK.SHUFFIELD@LOVES.COM

UTILITIES NOTIFICATION

AT LEAST TWO WORKING DAYS PRIOR TO COMMENCING CONSTRUCTION OPERATIONS IN AN AREA WHICH MAY INVOLVE UNDERGROUND UTILITY FACILITIES, THE CONTRACTOR SHALL CONTACT OHIO811 OUPS AT 8-1-1 OR 1-800-362-2764 (OHIO811.ORG), THE ENGINEER, THE REGISTERED UTILITY PROTECTION SERVICE AND THE OWNERS OF EACH UNDERGROUND UTILITY FACILITY SHOWN ON THE PLAN. THE OWNER OF THE UNDERGROUND FACILITY SHALL, WITHIN 48 HOURS (EXCLUDING SATURDAYS, SUNDAYS AND LEGAL HOLIDAYS), MARK THE LOCATION OF THE UNDERGROUND UTILITY FACILITIES IN THE CONSTRUCTION AREA IN SUCH A MANNER AS TO INDICATE THEIR COURSE AND THE APPROXIMATE DEPTH AT WHICH THEY WERE INSTALLED. THE MARKING OR LOCATING SHALL BE COORDINATED TO STAY APPROXIMATELY TWO DAYS AHEAD OF THE PLANNED CONSTRUCTION.

TEMPORARY TRAFFIC CONTROL

IT IS ESSENTIAL THAT THE ENGINEER HAVE THE AUTHORITY TO CONTROL THE PROGRESS OF THE WORK ON THE PROJECT IN ITS RELATION TO OBTAINING SAFE CONDITIONS. THIS INCLUDES THE AUTHORITY TO MODIFY CONDITIONS OR HALT WORK UNTIL APPLICABLE OR REMEDIAL SAFETY MEASURES ARE TAKEN. THE CONTRACTOR IS TO SUPPLY ALL TRAFFIC CONTROL, AS THE ENGINEER DEEMS NECESSARY. ALL TRAFFIC CONTROL DEVICES SHALL CONFORM TO THE APPLICABLE SPECIFICATIONS OF THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES. ALL WORK AND TRAFFIC CONTROL DEVICES SHALL BE IN ACCORDANCE WITH SECTION 614 OF THE ODOT STANDARD SPECIFICATIONS AND OTHER APPLICABLE PORTIONS OF THE ODOT SPECIFICATIONS, AS WELL AS THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

CLEARING AND GRUBBING

ALTHOUGH THERE ARE NO TREES OR STUMPS SPECIFICALLY MARKED FOR REMOVAL WITHIN THE LIMITS OF THE PROJECT, A LUMP SUM QUANTITY IS INCLUDED IN THE GENERAL SUMMARY FOR ITEM 201, CLEARING AND GRUBBING. ALL PROVISIONS AS SET FORTH IN THE SPECIFICATIONS UNDER THIS ITEM ARE INCLUDED IN THE LUMP SUM PRICE BIG FOR ITEM 201, CLEARING AND GRUBBING.

SURVEY DATA

PRIMARY PROJECT CONTROL MONUMENTS GOVERN ALL POSITIONING ON ODOT PROJECTS. SEE SHEET R2 OF THE PLANS FOR A TABLE CONTAINING PROJECT CONTROL INFORMATION. USE THE FOLLOWING PROJECT CONTROL, VERTICAL POSITIONING, AND HORIZONTAL POSITIONING PARAMETERS FOR ALL SURVEYING:

BASIS OF BEARINGS: BEARINGS ARE BASED UPON OHIO STATE PLANE SOUTH ZONE

PROJECT CONTROL

POSITIONING METHOD: ODOT REAL TIME NETWORK
MONUMENT TYPE: TYPE 'B'
VERTICAL POSITIONING
ORTHOMETRIC HEIGHT DATUM: NAVD88
GEOID: 18B

HORIZONTAL POSITIONING

REFERENCE FRAME: NAD83 (2011)
ELLIPSOID: GRS 80
COORDINATE SYSTEM: OHIO STATE PLANE SOUTH ZONE
PROJECTION: LAMBERT CONFORMAL CONIC
CENTRAL LATITUDE: N 40° 03' 28"
CENTRAL LONGITUDE: W 81° 03' 06"
PROJECTION SCALE FACTOR: 1.00000000

USE THE POSITIONING METHODS AND PRIMARY MONUMENT TYPE USED IN THE ORIGINAL SURVEY TO RESTORE ALL MONUMENTS RELATED TO PRIMARY PROJECT CONTROL THAT ARE DAMAGED OR DESTROYED BY CONSTRUCTION ACTIVITIES. RESTORE THE DAMAGED OR DESTROYED MONUMENTS IN ACCORDANCE WITH CMS 623.

UNITS ARE IN U.S. SURVEY FEET. USE THE FOLLOWING CONVERSION FACTOR: 1 METER=3.2808333333 U.S. SURVEY FEET

ITEM SPECIAL - MAILBOX SUPPORT

THIS WORK SHALL CONSIST OF FURNISHING AND ERECTING MAILBOX SUPPORTS AND ANY ASSOCIATED MOUNTING HARDWARE IN ACCORDANCE WITH PLAN DETAILS, AND ATTACHING AN OWNER-SUPPLIED MAILBOX AT LOCATIONS SPECIFIED IN THE PLAN, OR OTHERWISE ESTABLISHED BY THE ENGINEER. WOOD POSTS SHALL BE NOMINAL 4 INCHES BY 4 INCHES SQUARE OR 4.5 INCHES DIAMETER ROUND, AND CONFORM TO 710.14. STEEL POSTS SHALL BE NOMINAL PIPE SIZE 2 INCHES I.D., AND CONFORM TO AASHTO M 181. ALL HARDWARE INCLUDING BUT NOT LIMITED TO PLATES, SCREWS, BOLTS, AND ETC. SHALL BE COMMERCIAL-GRADE GALVANIZED STEEL. POSTS SHALL BE SET PER THE FIRST PARAGRAPH OF 606.03, AND SHALL IN NO INSTANCE BE ENCASED IN CONCRETE. SUPPORT HARDWARE SHALL ACCOMMODATE EITHER A SINGLE OR A DOUBLE MAILBOX INSTALLATION, AND NO MORE THAN TWO BOXES MAY BE MOUNTED ON A SINGLE POST. THE MAILBOX SHALL BE SECURELY AND NEATLY ATTACHED BY THE CONTRACTOR TO THE NEW SUPPORT. THE CONTRACTOR SHALL FURNISH ALL NECESSARY ATTACHMENT HARDWARE (NUTS, BOLTS, PLATES, SPACERS, AND WASHERS) AS NECESSARY TO ACCOMMODATE THE COMPLETE INSTALLATION. IN THE ABSENCE OF A NEW BOX SUPPLIED BY THE OWNER, THE CONTRACTOR SHALL SALVAGE THE EXISTING BOX AND PLACE IT ON THE NEW SUPPORT. DUE CARE SHALL BE EXERCISED IN SUCH AN OPERATION, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING OR REPLACING ANY BOX DAMAGED BY IMPROPER HANDLING ON HIS PART, AS JUDGED AND DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH THE LOCAL POST MASTER REGARDING THE TIMING OF THE MOVEMENT OF ANY MAILBOX TO A NEW LOCATION. PAYMENT UNDER THIS ITEM SHALL BE LIMITED TO FINAL PERMANENT INSTALLATIONS, TEMPORARY INSTALLATIONS SHALL BE IN ACCORDANCE WITH 107.10. HOWEVER, THE SAME MATERIAL AND SIZE LIMITATIONS AS FOR PERMANENT INSTALLATIONS SHALL APPLY. MAILBOX SUPPORTS, COMPLETE IN PLACE, WILL BE PAID FOR AT THE CONTRACT UNIT PRICE PER EACH, FOR ITEM SPECIAL MAILBOX SUPPORT SYSTEM, (SINGLE) (DOUBLE).

RESTORATION

THE CONTRACTOR SHALL PROVIDE FOR PROTECTION OF THE FOLLOWING:

EXISTING STREETS, DRIVEWAYS, SIDEWALKS, CURBS, GUTTERS, RE-SEEDING/RE-SODDING, REMOVAL OF TREES, RESTORATION OF AGRICULTURAL LAND, THE MAINTENANCE OF THE CONSTRUCTION AREA DURING PROGRESS OF THE WORK AND THE COMPLETE RESTORATION OF THE CONSTRUCTION AREA TO ITS ORIGINAL CONDITION AT THE COMPLETION OF THE WORK. THE CONTRACTOR SHALL CONTINUOUSLY CARRY ON WITH THE FINAL RESTORATION OF THE CONSTRUCTION AREA AFTER THE BACKFILLING IS COMPLETED, AND HE SHALL PROCEED TO RESTORE TO ITS ORIGINAL CONDITION ALL STREETS, DRIVEWAYS, SIDEWALKS, CURBS, GUTTERS, STRUCTURES, AND ALL AREAS THAT WERE DAMAGED, DISTURBED, OR OCCUPIED BY THE CONTRACTOR IN CONNECTION WITH ANY PHASE OF THE WORK.

PAVEMENTS, PAVEMENT MARKINGS, TREES, SHRUBS, FENCES, POLES, OR OTHER PROPERTY AND SURFACE STRUCTURES WHICH HAVE BEEN DAMAGED, REMOVED, OR DISTURBED BY THE CONTRACTOR, WHETHER DELIBERATELY OR THROUGH FAILURE TO CARRY OUT THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, STATE LAWS, MUNICIPAL ORDINANCES, THE SPECIFIC DIRECTION OF THE ENGINEER, OR THROUGH FAILURE TO EMPLOY USUAL AND REASONABLE SAFEGUARDS SHALL BE REPLACED AT THE EXPENSE OF THE CONTRACTOR.

ALL TEMPORARY SIGNS, SPRINKLER SYSTEMS, LANDSCAPING, ORNAMENTAL FENCING, ETC., LOCATED WITHIN THE PROJECT LIMITS MUST BE REPLACED OR RELOCATED UPON COMPLETION OF THE CONSTRUCTION.

ITEM 832: EROSION CONTROL

ALL EROSION AND SEDIMENTATION CONTROL SHALL BE PERFORMED ACCORDING TO: DETAIL PLANS; ACCORDING TO THE LATEST OHIO EPA AUTHORIZATION FOR CONSTRUCTION ACTIVITY UNDER THE "NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM" (NPDES); ANY AND ALL REQUIRED PERMITS, REPORTS, AND RELATED DOCUMENTS. SEE OHIO EPA PERMIT NO. OHR000006 FOR REGULATIONS. ALL CONTRACTORS AND SUBCONTRACTORS MUST BECOME FAMILIAR WITH ALL OF THE ABOVE. ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED IN THE FIELD BY THE INSPECTOR. THE CONTRACTOR IS RESPONSIBLE FOR ALL EROSION CONTROL MEASURES THAT ARE USED FOR WATER QUALITY. THE EROSION AND SEDIMENTATION CONTROL FACILITIES SHOWN ON THESE PLANS ARE TO BE CONSIDERED ADEQUATE BASIC REQUIREMENTS FOR THE ANTICIPATED SITE CONDITIONS. DURING CONSTRUCTION, DEVIATIONS FROM THIS PLAN MAY BE NECESSARY IN ORDER TO MAINTAIN WATER QUALITY. THE CONTRACTOR SHALL ADHERE TO ODOT'S SUPPLEMENTAL SPECIFICATION 832 FOR EROSION AND SEDIMENTATION CONTROL.

ASPHALT CONCRETE JOINT SEALER

THE CONTRACTOR SHALL APPLY LIQUID ASPHALT CONCRETE (PG64-22) TO THE VERTICAL FACE OF ALL EXISTING ASPHALT PRIOR TO THE PLACEMENT OF THE ADJOINING ASPHALT CONCRETE LIFT. PAYMENT FOR THIS WORK SHALL BE INCIDENTAL TO EACH RESPECTIVE ASPHALT CONCRETE COURSE OR BASE BEING PLACED.

BENCHING OF FOUNDATION SLOPES

ALTHOUGH CROSS-SECTIONS INDICATE SPECIFIC DIMENSIONS FOR PROPOSED BENCHING OF THE EMBANKMENT FOUNDATIONS IN CERTAIN AREAS, NO WAIVER OF THE SPECIFICATIONS IS INTENDED. BENCH ALL OTHER SLOPED EMBANKMENT AREAS AS SET FORTH IN SECTION 203.05 OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS (C&MS). NO ADDITIONAL PAYMENT WILL BE MADE FOR BENCHING REQUIRED UNDER THE PROVISIONS OF SECTION 203.05.

PART-WIDTH CONSTRUCTION

BECAUSE OF THE NECESSITY TO BUILD THIS PROJECT UNDER TRAFFIC AND TO CONSTRUCT THE FULL PAVEMENT WIDTH IN STAGES, EXERCISE CARE TO PREVENT THE CONSTRUCTION OF A BUTT JOINT IN THE BASE COURSES. LAP LONGITUDINAL JOINTS AS SHOWN ON STANDARD CONSTRUCTION DRAWING BP-3.1.

CONNECTION BETWEEN EXISTING AND PROPOSED GUARDRAIL

WHEN IT IS NECESSARY TO SPLICE PROPOSED GUARDRAIL TO EXISTING GUARDRAIL, ONLY THE EXISTING GUARDRAIL SHALL BE CUT, DRILLED, OR PUNCHED. THE CONNECTION SHALL BE MADE USING A W-BEAM, BEAM SPLICE AS SHOWN IN AASHTO M 180-12, EXCEPT THE BEAM WASHERS ARE NOT TO BE USED. PAYMENT SHALL BE INCLUDED IN THE CONTRACT PRICE FOR THE RESPECTIVE GUARDRAIL ITEMS.

ITEM 606 - ANCHOR ASSEMBLY, MGS TYPE E

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING ANY OF THE GUARDRAIL END TERMINALS FOR TYPE MGS GUARDRAIL AS LISTED ON ROADWAY ENGINEERING'S WEB PAGE UNDER ROADSIDE SAFETY DEVICES FOR APPROVED GUARDRAIL END TREATMENTS. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS, IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. THE FACE OF THE TYPE E IMPACT HEAD SHALL BE COVERED WITH REBOUNDABLE RETROREFLECTIVE SHEETING, PER CMS 730.191. REFER TO THE MANUFACTURER'S INSTRUCTIONS REGARDING THE INSTALLATION OF, AND THE GRADING AROUND THE FOUNDATION TUBES AND GROUND STRUT. THE TOP OF ANY FOUNDATION TUBE SHOULD BE LESS THAN 4 INCHES ABOVE THE GROUND. THE PLACEMENT OF THE FOUNDATION TUBES SHOULD BE AN APPROPRIATE DEPTH BELOW THE LEVEL LINE IN ORDER TO MAINTAIN THE FINISHED GUARDRAIL HEIGHT OF 31 INCHES FROM THE EDGE OF THE SHOULDER. ON-SITE GRADING IS REQUIRED IF THE TOP OF THE FOUNDATION TUBES OR TOP OF THE GROUND STRUT DOES PROJECT MORE THAN 4 INCHES ABOVE THE GROUND LINE. PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID FOR ITEM 606, ANCHOR ASSEMBLY, MGS TYPE E, EACH, AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT A COMPLETE AND FUNCTIONAL ANCHOR ASSEMBLY SYSTEM, INCLUDING ALL RELATED TRANSITIONS, REFLECTIVE SHEETING, HARDWARE, GRADING, EMBANKMENT AND EXCAVATION NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

DATUM NOTES:

HORIZONTAL DATUM:
OHIO STATE PLANE COORDINATE SYSTEM, OHIO SOUTH ZONE, NAD83 (2011) AS DETERMINED BY OPUS POST PROCESSED STATIC SESSION.

VERTICAL DATUM:
NAVD 88 AS DETERMINED BY OPUS POST PROCESSED STATIC SESSIONS USING GEOID 18 (CONUS)

UTILITY DISCLAIMER:

THE UTILITIES SHOWN HEREON HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION OUPS #A218001060 AND/OR EXISTING DRAWINGS. THE SURVEYOR MAKES NO GUARANTEE THAT THE UTILITIES LOCATED HERE ON COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UTILITIES LOCATED ARE IN THE EXACT LOCATION INDICATED ALTHOUGH HE DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.

DATE OF FIELD SURVEY: AUGUST 9, 2022
DATE OF SURVEY MAP: APRIL 12, 2023

REFERENCE DRAWINGS:

ORIGINAL SURVEY 1 OF 1	ALTANSPS LAND TITLE SURVEY BY CESCO INC. 175 MONTROSE WEST AVE. ST. SUITE 400 COPELY, OH 44321 PHONE: (330) 665-0660 CONTACT: STEVEN CLUTTER
ORIGINAL PLANS 1 OF 1	PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES BY TERRACON DATED: 4/08/24 4685 SOUTH ASH AVENUE, SUITE H-4 TEMPE, AZ 85282 PHONE: (480) 897-8200

ODOT ITEM 408 - PRIME COAT, AS PER PLAN

THE CONTRACTOR SHALL APPLY ONE COAT OF MC-70 (AS PER SECTION 702) AT A RATE OF 0.40 GAL/SY TO THE COMPLETED AGGREGATE SHOULDER AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHALL PROVIDE A SHIELD TO PREVENT THE SPRAYING OR DRIFTING OF LIQUID BITUMINOUS MATERIAL ONTO THE EDGE OF PAVEMENT OR EDGE LINE. THE ATTENTION OF THE CONTRACTOR IS DIRECTED TO 107.10 OF THE SPECIFICATIONS.

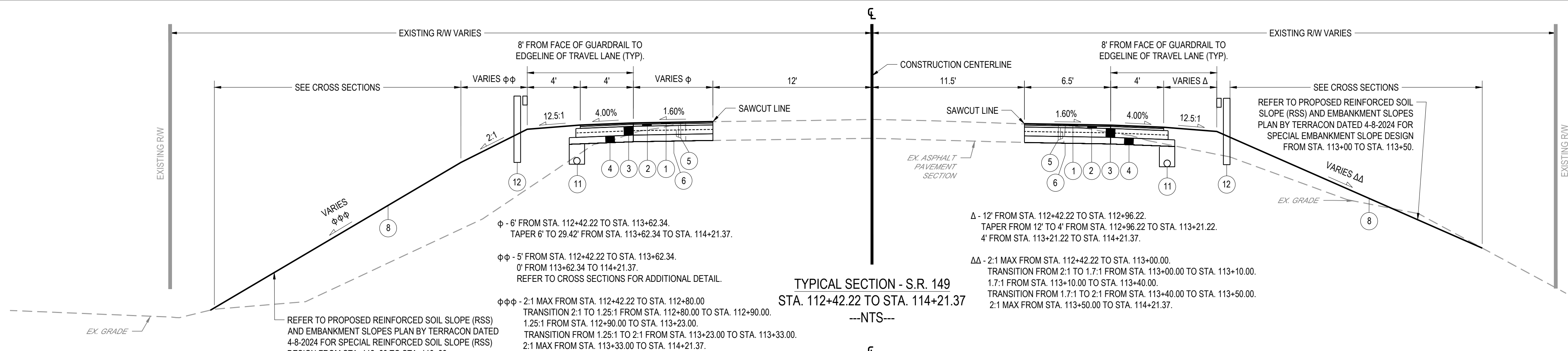
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GENERAL NOTES
S.R. 149

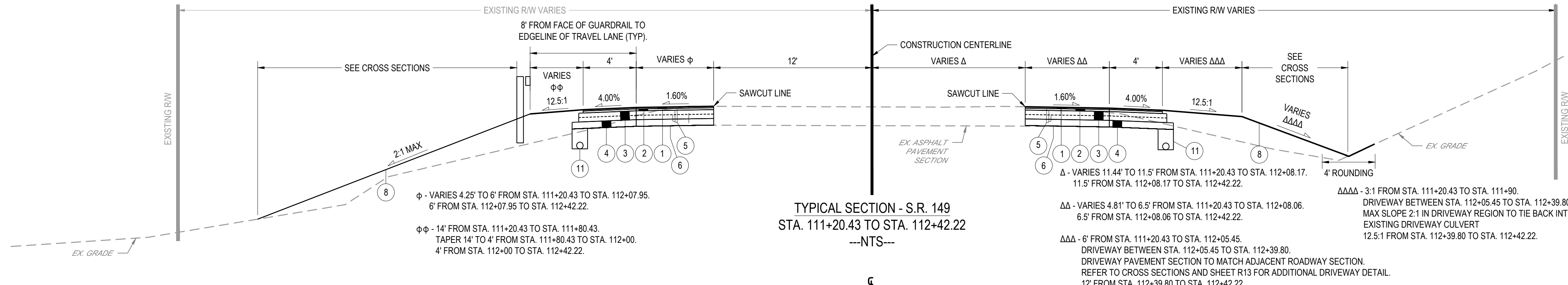
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R3
R31

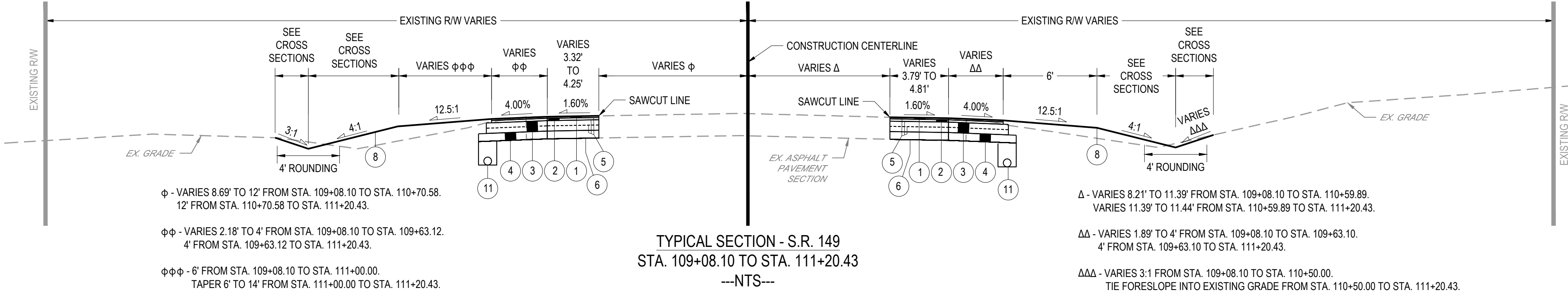
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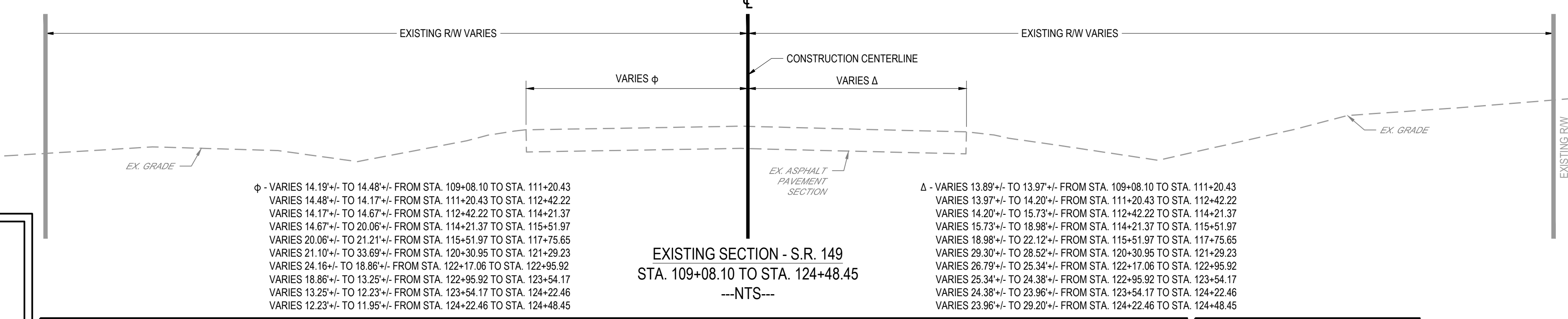
TYPICAL SECTION - S.R. 149
STA. 112+42.22 TO STA. 114+21.37
---NTS---



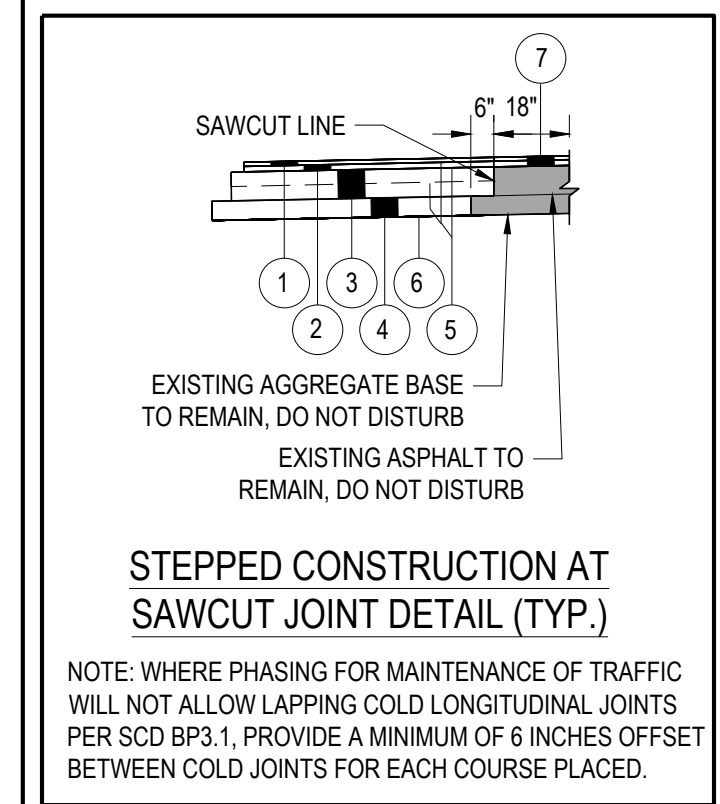
TYPICAL SECTION - S.R. 149
STA. 111+20.43 TO STA. 112+42.22
---NTS---



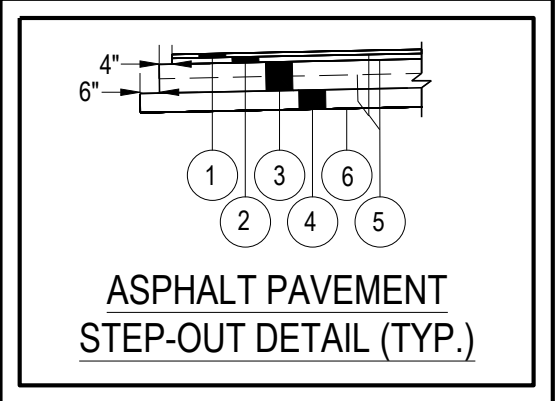
TYPICAL SECTION - S.R. 149
STA. 109+08.10 TO STA. 111+20.43
---NTS---



EXISTING SECTION - S.R. 149
STA. 109+08.10 TO STA. 124+48.45
---NTS---



LEGEND	
1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)
2	ODOT ITEM 441 - 1-3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)
3	ODOT ITEM 301 - 8" ASPHALT CONCRETE BASE, PG64-22 (TWO 4" LIFTS, ITEM 407 BETWEEN LIFT)
4	ODOT ITEM 304 - 6" AGGREGATE BASE
5	ODOT ITEM 407 - TACK COAT
6	ODOT ITEM 204 - SUBGRADE COMPACTION
7	ODOT ITEM 254 - 3" PAVEMENT PLANING (MILL OF EX. SURFACE AND INTERMEDIATE COURSE)
8	ODOT ITEM 659 - SEEDING AND MULCHING
9	ODOT ITEM 609 - ODOT TYPE 2 COMBINATION CURB AND GUTTER PER BP-5.1
10	ODOT ITEM 411 - 6" STABILIZED CRUSHED AGGREGATE WITH ODOT ITEM 408 - PRIME COAT, AS PER PLAN - SEE ITEM 408 NOTE
11	ODOT ITEM 605 - 6" BASE PIPE UNDERDRAIN WITH GEOTEXTILE FABRIC, TYPE A, 18" DEPTH TO PIPE BOTTOM FROM SUBGRADE (TYP). DEPTH VARIES AT SELECTED LOCATIONS. SEE CROSS SECTIONS FOR UNDERDRAIN ELEVATIONS/OFFSET.
12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
---	EXISTING GRADE/PAVEMENT SECTION



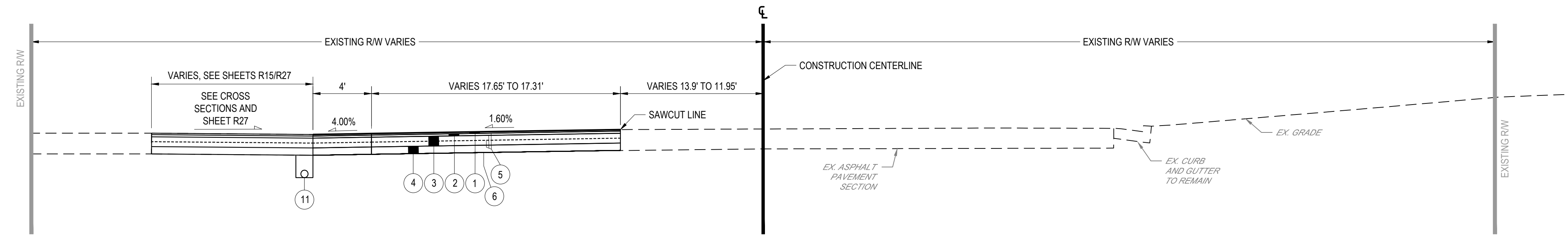
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TYPICAL SECTIONS
S.R. 149

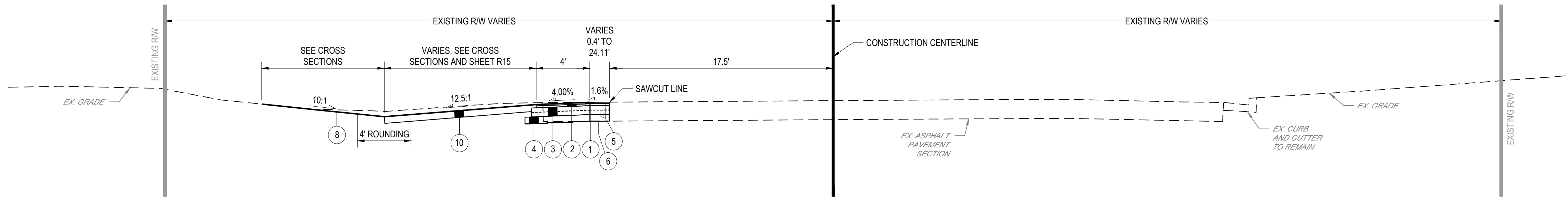
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R4
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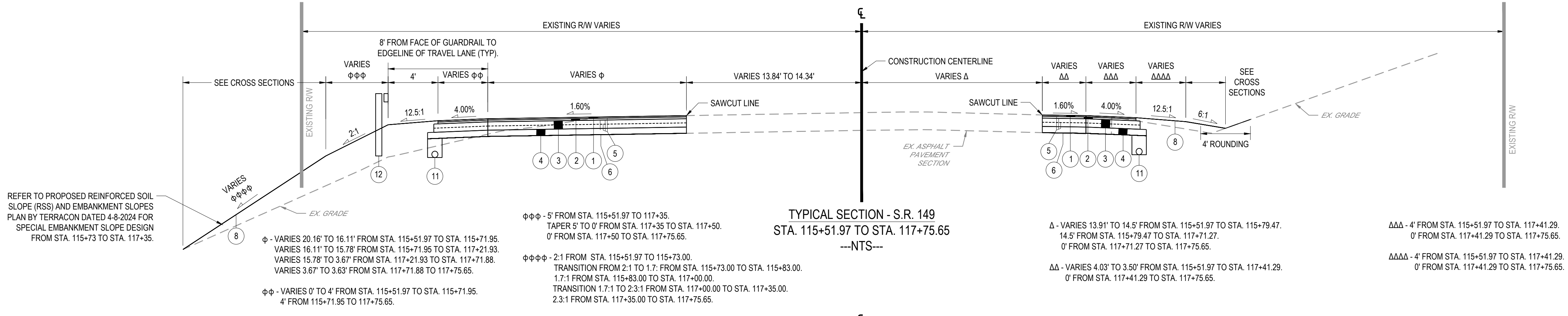
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TYPICAL SECTION - S.R. 149
STA. 122+17.06 TO STA. 122+95.92
---NTS---

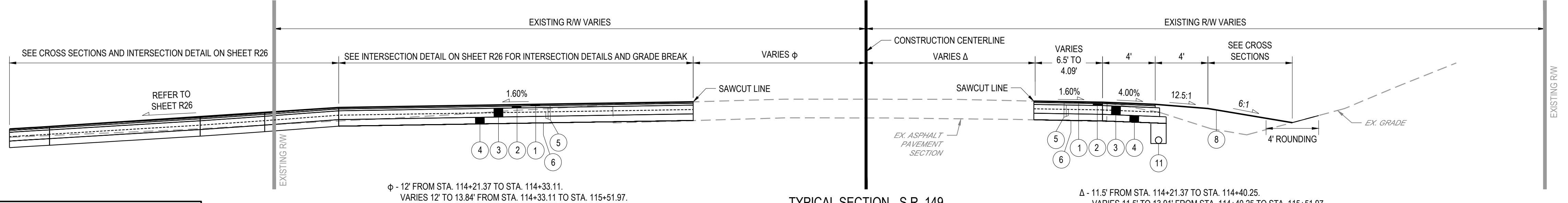


TYPICAL SECTION - S.R. 149
STA. 120+30.95 TO STA. 121+29.23
---NTS---

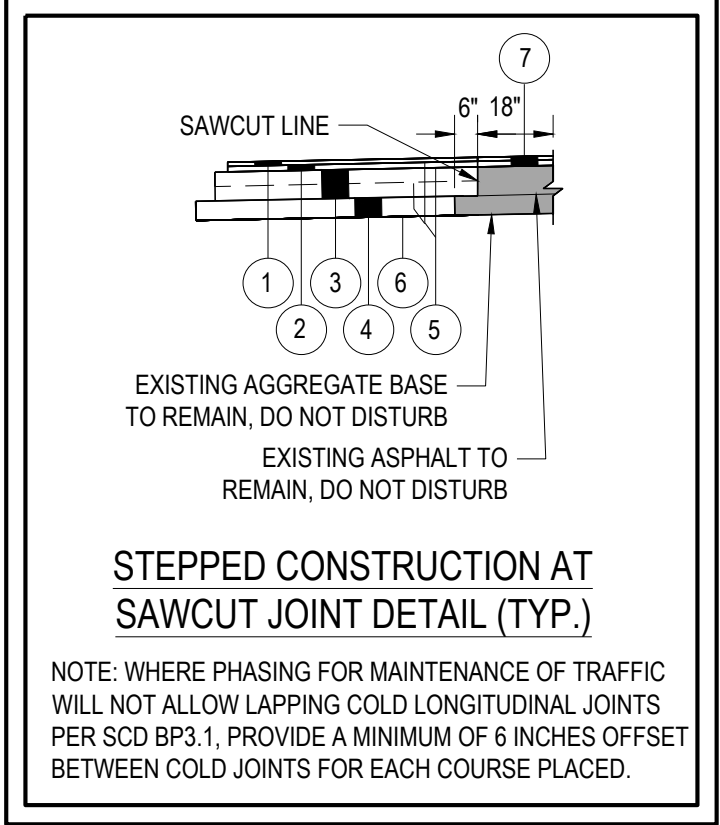


TYPICAL SECTION - S.R. 149
STA. 115+51.97 TO STA. 117+75.65
---NTS---

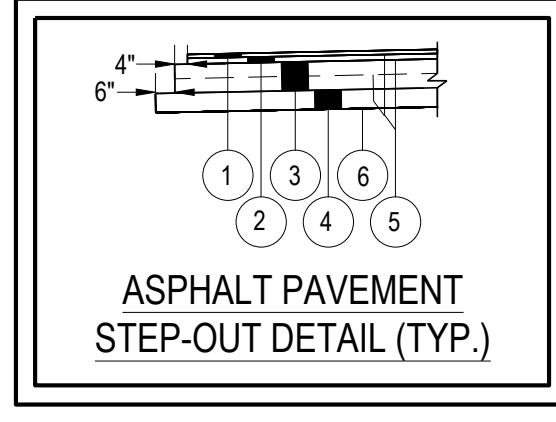
REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 4-8-2024 FOR SPECIAL EMBANKMENT SLOPE DESIGN FROM STA. 115+73 TO STA. 117+35.



TYPICAL SECTION - S.R. 149
STA. 114+21.37 TO STA. 115+51.97
---NTS---



LEGEND	
1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)
2	ODOT ITEM 441 - 1-3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)
3	ODOT ITEM 301 - 8" ASPHALT CONCRETE BASE, PG64-22 (TWO 4" LIFTS, ITEM 407 BETWEEN LIFT)
4	ODOT ITEM 304 - 6" AGGREGATE BASE
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12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
---	EXISTING GRADE/PAVEMENT SECTION



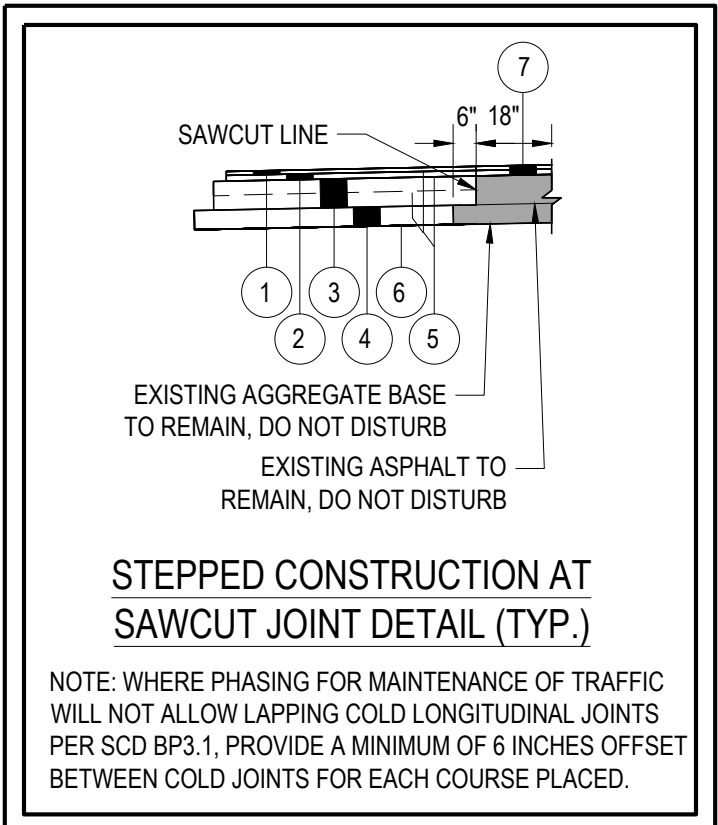
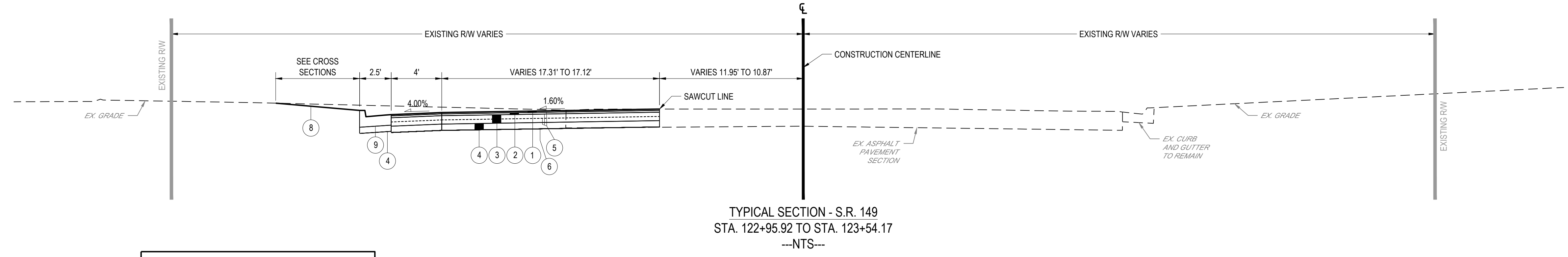
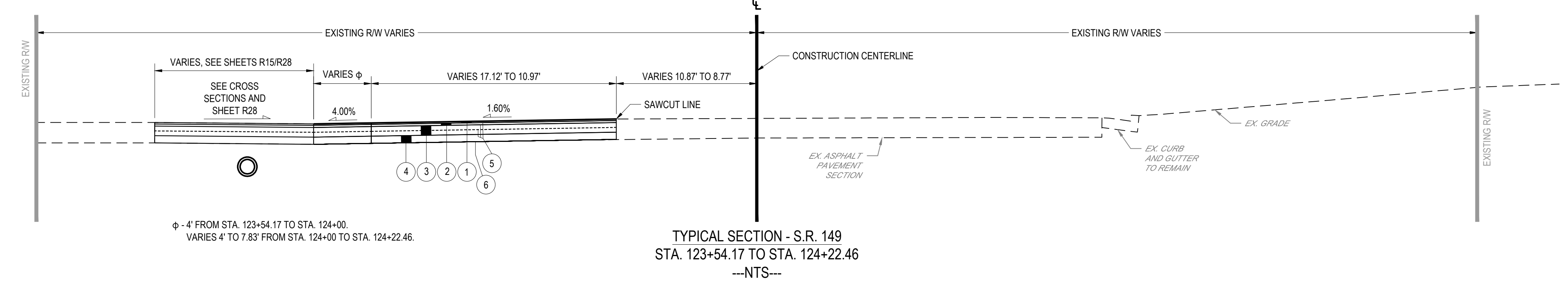
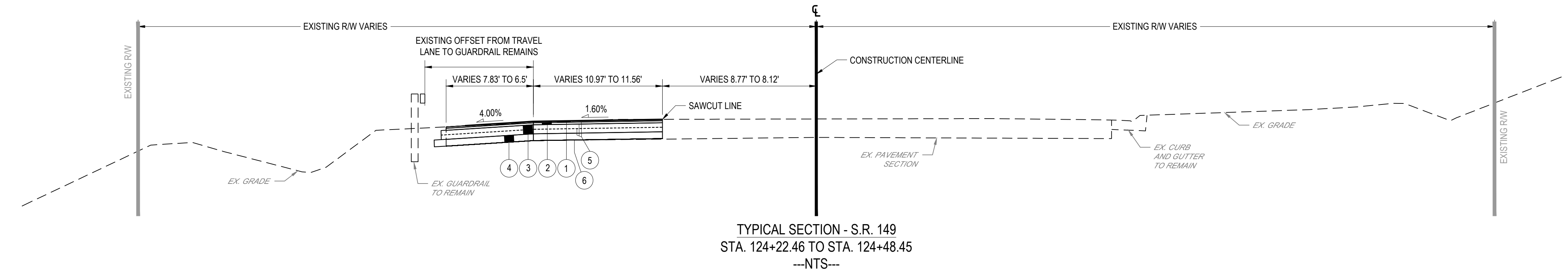
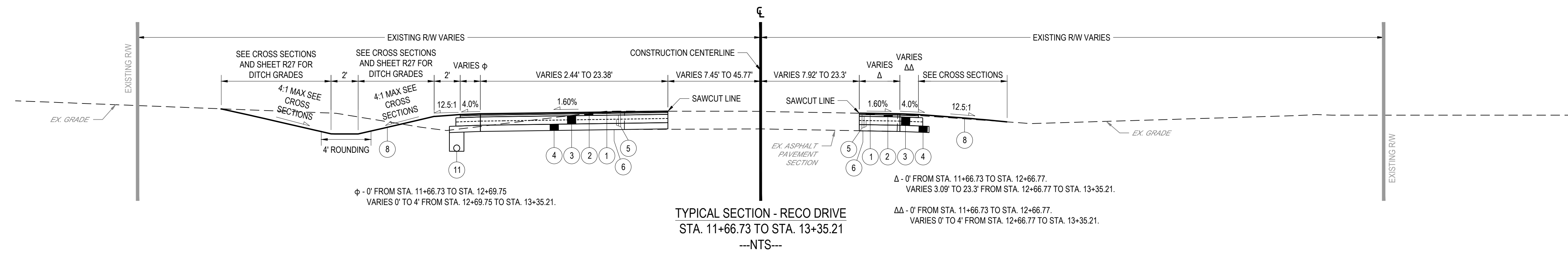
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TYPICAL SECTIONS
S.R. 149

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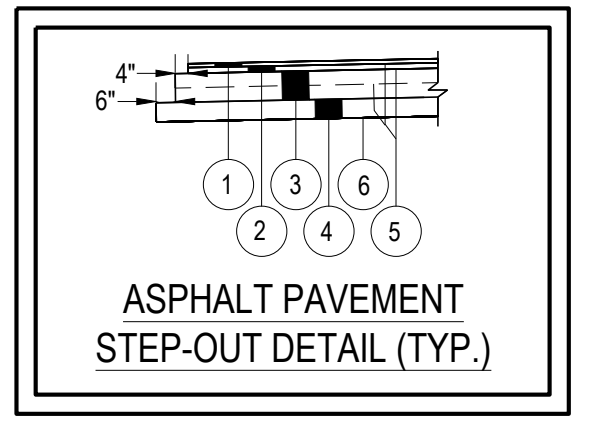
R5
R31

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LEGEND

1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)	8	ODOT ITEM 659 - SEEDING AND MULCHING
2	ODOT ITEM 441 - 1-3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)	9	ODOT ITEM 609 - ODOT TYPE 2 COMBINATION CURB AND GUTTER PER BP-5.1
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5	ODOT ITEM 407 - TACK COAT	12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
6	ODOT ITEM 204 - SUBGRADE COMPACTION	---	EXISTING GRADE/PAVEMENT SECTION
7	ODOT ITEM 254 - 3" PAVEMENT PLANING (MILL OF EX. SURFACE AND INTERMEDIATE COURSE)		



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TYPICAL SECTIONS
S.R. 149

BEL-149-23.44

R6
R31

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MAINTAINING TRAFFIC

THE CONTRACTOR SHALL PROVIDE, ERECT, AND MAINTAIN SIGNS (IN PROPER POSITION, CLEAN AND LEGIBLE, AND IN GOOD WORKING CONDITION) AND REMOVE ALL LIGHTS, SIGNS, CONES, DRUMS, AND ANY OTHER TRAFFIC CONTROL DEVICES NECESSARY FOR THE MAINTENANCE OF TRAFFIC ACCORDING TO THESE PLAN NOTES AND DETAILS.

NO CHANGES IN TRAFFIC CONTROL PATTERNS SHALL BE MADE BETWEEN THE HOURS OF 6:00 AM TO 9:00 PM MONDAY THROUGH FRIDAY, 8:00 AM-8:00 PM SATURDAY, AND 8:00 AM-8:00 PM SUNDAY; IN ADDITION, LANE CLOSURES ARE NOT PERMITTED 6:00 AM TO 9:00 PM MONDAY THROUGH FRIDAY, 8:00 AM-8:00 PM SATURDAY, AND 8:00 AM-8:00 PM SUNDAY. REFER TO LANE VALUE CONTRACT TABLE.

LANES OPEN DURING HOLIDAYS AND SPECIAL EVENTS

NO WORK SHALL BE PERFORMED AND THE SAME NUMBER OF LANES AS WERE AVAILABLE AT THE START OF THE PROJECT SHALL BE OPEN TO TRAFFIC DURING THE FOLLOWING DESIGNATED HOLIDAYS OR SPECIAL EVENTS:

HOLIDAYS:

NEW YEAR'S (OBSERVED), TOTAL SOLAR ECLIPSE (4/8/24), MEMORIAL DAY, FOURTH OF JULY (OBSERVED), LABOR DAY, GENERAL/REGULAR ELECTION DAY (NOV), THANKSGIVING, CHRISTMAS (OBSERVED).

THE PERIOD OF TIME THAT THE LANES ARE TO BE OPEN DEPENDS ON THE DAY OF THE WEEK ON WHICH THE HOLIDAY OR SPECIAL EVENT FALLS. THE FOLLOWING SCHEDULE SHALL BE USED TO DETERMINE THIS PERIOD:

DAY OF HOLIDAY OR SPECIAL EVENT TIME ALL LANES MUST BE OPEN TO TRAFFIC

SUNDAY	12:00 NOON FRIDAY THROUGH 6:00 AM MONDAY
MONDAY	12:00 NOON FRIDAY THROUGH 6:00 AM TUESDAY
MONDAY (TOTAL SOLAR ECLIPSE)	12:00 NOON FRIDAY THROUGH 6:00 AM WEDNESDAY
TUESDAY	12:00 NOON MONDAY THROUGH 6:00 AM WEDNESDAY
TUESDAY (GEN./REG. ELECTION)	5:00 AM TUESDAY THROUGH 12:00 AM WEDNESDAY
WEDNESDAY	12:00 NOON TUESDAY THROUGH 6:00 AM THURSDAY
THURSDAY	12:00 NOON WEDNESDAY THROUGH 6:00 AM FRIDAY
THANKSGIVING	6:00 AM WEDNESDAY THROUGH 6:00 AM MONDAY
FRIDAY	12:00 NOON THURSDAY THROUGH 6:00 AM MONDAY
SATURDAY	12:00 NOON FRIDAY THROUGH 6:00 AM MONDAY

DURING THE SAME PERIODS, MAINTAIN PEDESTRIAN ACCESS IF PEDESTRIAN ACCESS WAS PRESENT PRIOR TO CONSTRUCTION.

NO EXTENSIONS OF TIME SHALL BE GRANTED FOR DELAYS IN MATERIAL DELIVERIES, UNLESS SUCH DELAYS ARE INDUSTRY-WIDE, OR FOR LABOR STRIKES, UNLESS SUCH STRIKES ARE AREA-WIDE.

SHOULD THE CONTRACTOR FAIL TO MEET ANY OF THESE REQUIREMENTS, THE CONTRACTOR SHALL BE ASSESSED A DISINCENTIVE IN THE AMOUNT OF \$6,000/HR THE ABOVE DESCRIBED LANE CLOSURE RESTRICTIONS ARE VIOLATED.

LENGTH AND DURATION OF LANE CLOSURES AND RESTRICTIONS SHALL BE AT THE APPROVAL OF THE ENGINEER. IT IS THE INTENT TO MINIMIZE THE IMPACT TO THE TRAVELING PUBLIC. LANE CLOSURES OR RESTRICTIONS OVER SEGMENTS OF THE PROJECT IN WHICH NO WORK IS ANTICIPATED WITHIN A REASONABLE TIME FRAME, AS DETERMINED BY THE ENGINEER, SHALL NOT BE PERMITTED. THE LEVEL OF UTILIZATION OF MAINTENANCE OF TRAFFIC DEVICES SHALL BE COMMENSURATE WITH THE WORK IN PROGRESS.

BEFORE THE WORK BEGINS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE NAMES AND TELEPHONE NUMBERS OF PERSONS WHO CAN BE CONTACTED TWENTY-FOUR (24) HOURS PER DAY BY ODOT AND ALL INTERESTED POLICE AGENCIES. THIS PERSON OR PERSONS SHALL BE RESPONSIBLE FOR PLACING OR REPLACING NECESSARY TRAFFIC CONTROL DEVICES IN ACCORDANCE WITH THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN TRAFFIC AT ALL TIMES. IT IS THE INTENTION TO PERFORM THE REQUIRED WORK WITH THE LEAST INCONVENIENCE TO AND THE MAXIMUM SAFETY OF THE WORKERS AND THE TRAVELING PUBLIC OCCASIONALLY, MODIFICATIONS IN TRAFFIC CONTROLS OR WORKING CONDITIONS MAY BE REQUIRED IN ORDER TO EXPEDITE SAFE TRAFFIC MOVEMENT AND TO PROMOTE WORKER SAFETY.

NOTIFICATION OF TRAFFIC RESTRICTIONS

THROUGHOUT THE DURATION OF THE PROJECT, THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER IN WRITING OF ALL TRAFFIC RESTRICTIONS AND UPCOMING MAINTENANCE OF TRAFFIC CHANGES. THE CONTRACTOR SHALL ENSURE THE WRITTEN NOTIFICATION IS SUBMITTED IN A TIMELY MANNER TO ALLOW THE PROJECT ENGINEER TO MEET THE REQUIRED TIME FRAMES SET FORTH IN THE TABLE BELOW TO INFORM THE SPECIAL HAULING PERMITS SECTION (HAULING.PERMITS@DOT.OHIO.GOV) AND THE DISTRICT PUBLIC INFORMATION OFFICE (PIO). THIS NOTIFICATION SHALL BE RECEIVED BY THE PROJECT ENGINEER PRIOR TO THE PHYSICAL SETUP OF ANY APPLICABLE SIGNS OR MESSAGE BOARDS. INFORMATION SHOULD INCLUDE, BUT IS NOT LIMITED TO, ALL CONSTRUCTION ACTIVITIES THAT IMPACT OR INTERFERE WITH TRAFFIC AND SHALL LIST THE SPECIFIC LOCATION, TYPE OF WORK, ROAD STATUS, DATE AND TIME OF RESTRICTION, DURATION OF RESTRICTION, NUMBER OF LANES MAINTAINED, NUMBER OF LANES CLOSED, MINIMUM VERTICAL CLEARANCE, MINIMUM WIDTH OF DRIVABLE PAVEMENT, DETOUR ROUTES, IF APPLICABLE, AND ANY OTHER INFORMATION REQUESTED BY THE PROJECT ENGINEER.

NOTIFICATION OF TRAFFIC RESTRICTIONS TIME TABLE

ITEM	DURATION OF CLOSURE	NOTICE DUE TO PERMITS & PIO
RAMP &	> 2 WEEKS	21 CALENDAR DAYS PRIOR TO CLOSURE
ROAD	> 12 HOURS & < 2 WEEKS	14 CALENDAR DAYS PRIOR TO CLOSURE
CLOSURES	< 12 HOURS	4 BUSINESS DAYS PRIOR TO CLOSURE
LANE	> 2 WEEKS	14 CALENDAR DAYS PRIOR TO CLOSURE
CLOSURES & RESTRICTIONS	< 2 WEEKS	5 BUSINESS DAYS PRIOR TO CLOSURE
START OF CONSTRUCTION & TRAFFIC PATTERN CHANGES	N/A	14 CALENDAR DAYS PRIOR TO IMPLEMENTATION

ANY UNFORESEEN CONDITIONS NOT SPECIFIED IN THE PLANS REQUIRING TRAFFIC RESTRICTIONS SHALL ALSO BE REPORTED TO THE PROJECT ENGINEER USING THE NOTIFICATION TIME TABLE.

WORK SITE LIGHTING

FLOODLIGHTING OF THE WORK SITE FOR OPERATIONS CONDUCTED DURING NIGHTTIME PERIODS SHALL BE ACCOMPLISHED SO THAT THE LIGHTS DO NOT CAUSE GLARE TO THE DRIVERS ON THE ROADWAY. TO ENSURE THE ADEQUACY OF FLOODLIGHT PLACEMENT, THE CONTRACTOR, AND THE ENGINEER SHALL DRIVE THROUGH THE WORK SITE EACH NIGHT WHEN THE LIGHTING IS IN PLACE AND OPERATIVE PRIOR TO COMMENCING ANY WORK. IF GLARE IS DETECTED, THE LIGHT PLACEMENT AND SHIELDING SHALL BE ADJUSTED TO THE SATISFACTION OF THE ENGINEER BEFORE WORK PROCEEDS. PAVEMENT FOR ALL LABOR, EQUIPMENT, AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACTOR PRICE FOR ITEM 614, MAINTAINING TRAFFIC.

MUD, DIRT AND DEBRIS

THE TRACKING OR SPILLAGE OF MUD, DIRT OR DEBRIS UPON STATE HIGHWAYS IS PROHIBITED AND ANY SUCH OCCURRENCE SHALL BE CLEANED UP IMMEDIATELY BY THE CONTRACTOR.

PERSONAL PROTECTIVE EQUIPMENT

ODOT REQUIRES ALL CONTRACTORS' PERSONNEL TO WEAR THE CORRECT PPE WHILE WITHIN ODOT RIGHT-OF-WAY. ALL VEHICLES SHALL HAVE THE CORRECT SAFETY EQUIPMENT ALSO.

THE CONTRACTOR SHALL PROVIDE, ERECT AND MAINTAIN SIGNS AND SIGN SUPPORTS, AS DETAILED IN THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

ALL VEHICLES, EQUIPMENT AND WORKERS AND THEIR ACTIVITIES ARE RESTRICTED TO ONE SIDE OF THE PAVEMENT UNLESS OTHERWISE APPROVED BY THE ENGINEER. CONSTRUCTION VEHICLES SHALL ALWAYS MOVE WITH AND NOT AGAINST THE FLOW OF TRAFFIC. CONSTRUCTION VEHICLES AND EQUIPMENT SHALL ENTER AND LEAVE WORK AREAS IN A MANNER WHICH WILL NOT BE HAZARDOUS TO, OR INTERFERE WITH THE NORMAL FLOW OF TRAFFIC. PERSONAL VEHICLES ARE NOT PERMITTED IN THE RIGHT-OF-WAY EXCEPT IN SPECIFIC AREAS DESIGNATED BY THE ENGINEER.

MAINTAINING TRAFFIC (SIGNS AND BARRICADES)

THE CONTRACTOR SHALL PROVIDE, ERECT AND MAINTAIN SIGNS AND SIGN SUPPORTS, AS DETAILED IN THE OHIO MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES, AND TYPE III BARRICADES AS DIRECTED IN CONSTRUCTION SEQUENCE.

TEMPORARY PAVEMENT WEDGE

TEMPORARY PAVEMENT WEDGES SHALL BE PROVIDED AT ALL TIMES WHERE TRAFFIC IS REQUIRED TO TRAVEL FROM OR ONTO A PAVEMENT SURFACE OF A DIFFERENT ELEVATION. THE MINIMUM SLOPE OF THE TEMPORARY PAVEMENT WEDGE SHALL BE 3:1 ALONG LONGITUDINAL JOINTS AND 120:1 AT TRANSVERSE JOINTS. THESE WEDGES SHALL BE REMOVED PRIOR TO PLACING THE SPECIFIED PAVEMENT COURSE.

PLACEMENT OF ASPHALT CONCRETE

TWO-WAY TRAFFIC SHALL BE MAINTAINED AT ALL TIMES EXCEPT THAT ONE WAY TRAFFIC WILL BE PERMITTED FOR MINIMUM PERIODS OF TIME CONSISTENT WITH THE REQUIREMENTS OF THE SPECIFICATIONS FOR PROTECTION OF COMPLETED ASPHALT COURSES.

TRENCH FOR WIDENING

TRENCH FOR EXCAVATION FOR BASE WIDENING SHALL BE FOR ONLY ONE SIDE OF THE PAVEMENT AT A TIME. THE OPEN TRENCH SHALL BE ADEQUATELY MAINTAINED AND PROTECTED WITH DRUMS OR BARRICADES AT ALL TIMES. PLACEMENT OF PROPOSED SUBBASE AND BASE MATERIAL SHALL FOLLOW CLOSELY AS POSSIBLE BEHIND EXCAVATION OPERATIONS. THE LENGTH OF WIDENING TRENCH WHICH IS OPEN AT ANY ONE TIME SHALL BE HELD TO A MINIMUM AND SHALL AT ALL TIMES BE SUBJECT TO APPROVAL BY THE ENGINEER.

THE CONTRACTOR SHALL ENSURE THAT THE TRENCH FOR WIDENING HAS A VERTICAL DROP OF NO MORE THAN 5 INCHES. THE TRENCH CAN BE BACKFILLED BY A COMPACTED AGGREGATE WEDGE. THE MINIMUM SLOPE OF THE TEMPORARY COMPACTED AGGREGATE WEDGE SHALL BE 3:1 ALONG THE EXISTING PAVEMENT AND PROTECTED WITH DRUMS AT ALL TIMES.

OVERNIGHT TRENCH CLOSING

THE BASE WIDENING SHALL BE COMPLETED TO A DEPTH OF NO MORE THAN 5 INCHES BELOW THE EXISTING PAVEMENT BY THE END OF EACH WORK DAY. NO TRENCH SHALL BE LEFT OPEN OVERNIGHT EXCEPT FOR A SHORT LENGTH (25 FEET OR LESS) OF A WORK SECTION AT THE END OF THE DAY. IN CASE WORK MUST BE SUSPENDED BECAUSE OF INCLEMENT WEATHER OR OTHER REASONS, THE TRENCH FOR THE UNCOMPLETED BASE WIDENING SHALL BE BACKFILLED AT THE DIRECTION OF THE ENGINEER.

DROPOFFS IN WORK ZONE

THE DROPOFF ADJACENT TO THE TRAVELED LANE SHALL MEET THE CRITERIA OUTLINED ODOT STANDARD DRAWING MT-101.90. NO ADDITIONAL COMPENSATION SHALL BE MADE FOR MATERIALS, LABOR OR EQUIPMENT NECESSARY TO MEET THE REQUIREMENTS OF MT-101.90.

ITEM 614, LAW ENFORCEMENT OFFICER (WITH PATROL CAR) FOR ASSISTANCE DURING CONSTRUCTION OPERATIONS

USE OF LAW ENFORCEMENT OFFICERS (LEOS) BY CONTRACTORS OTHER THAN THE USES SPECIFIED BELOW WILL NOT BE PERMITTED AT PROJECT COST. LEOS SHOULD NOT BE USED WHERE THE OMUTCD INTENDS THAT FLAGGERS BE USED.

IN ADDITION TO THE REQUIREMENTS OF C&MS 614 AND THE OMUTCD, A UNIFORMED LEO WITH AN OFFICIAL PATROL CAR (CAR WITH TOP-MOUNTED EMERGENCY FLASHING LIGHTS AND COMPLETE MARKINGS OF THE APPROPRIATE LAW ENFORCEMENT AGENCY) SHALL BE PROVIDED FOR THE FOLLOWING TRAFFIC CONTROL TASKS:

- DURING THE ENTIRE ADVANCE PREPARATION AND CLOSURE SEQUENCE WHERE COMPLETE BLOCKAGE OF TRAFFIC IS REQUIRED.
- DURING A TRAFFIC SIGNAL INSTALLATION WHEN IMPACTING THE NORMAL FUNCTION OF THE SIGNAL OR THE FLOW OF TRAFFIC, OR WHEN TRAFFIC NEEDS TO BE DIRECTED THROUGH AN ENERGIZED TRAFFIC SIGNAL CONTRARY TO THE SIGNAL DISPLAY (E.G., DIRECTING MOTORISTS THROUGH A RED LIGHT).

IN ADDITION TO THE REQUIREMENT OF C&MS 614 AND THE OMUTCD, A UNIFORMED LEO WITH AN OFFICIAL PATROL CAR (CAR WITH TOP-MOUNTED EMERGENCY FLASHING LIGHTS AND COMPLETE MARKINGS OF THE APPROPRIATE LAW ENFORCEMENT AGENCY) SHOULD BE PROVIDED FOR THE FOLLOWING TRAFFIC CONTROL TASKS AS APPROVED BY THE ENGINEER:

- FOR LANE CLOSURES: DURING INITIAL SET-UP PERIODS, TEAR DOWN PERIODS, SUBSTANTIAL SHIFTS OF A CLOSURE POINT OR WHEN NEW LANE CLOSURE ARRANGEMENTS ARE INITIATED FOR LONG-TERM LANE CLOSURES/SHIFTS (FOR THE FIRST AND LAST DAY OF MAJOR CHANGES IN TRAFFIC CONTROL SETUP).
- FOR OPERATIONS WITHOUT POSITIVE PROTECTION OCCURRING WITHIN 10 FEET OF AN OPEN TRAVELED LANE THAT MEET ALL OF THE FOLLOWING CRITERIA:
 - ON A MULTI-LANE DIVIDED INTERSTATE, OTHER FREEWAY OR EXPRESSWAY; AND
 - AN AUTHORIZED SPEED LIMIT OF 45 MPH OR GREATER THAT IS IN EFFECT AT THE TIME OF OPERATION; AND,
 - AADT OF 50,000 (OR AADT OF 30,000 WITH 25% OR HIGHER PERCENT TRUCKS)

"WITHOUT POSITIVE PROTECTION" MEANS USE OF DRUMS, CONES, SHADOW VEHICLE, ETC, WITHOUT PROTECTION FROM PORTABLE BARRIER OR OTHER RIGID BARRIER ALONG THE WORK AREA. THIS PHRASE DOES NOT APPLY TO CASES WHERE POSITIVE PROTECTION IS REQUIRED. MOBILE OPERATIONS ARE REGARDED AS "WITHOUT POSITIVE PROTECTION". FOR WORK ZONES USING A COMBINATION OF BARRIER AND TEMPORARY TRAFFIC CONTROL DEVICES (CONES, DRUMS, ETC), THE DESIGNATION SHALL BE BASED UPON THE TYPE OF DEVICES USED IN THE AREA THAT WORKERS ARE LOCATED.

IF MULTIPLE ACTIVE LOCALIZED QUALIFYING WORK AREAS OCCUR WITHOUT POSITIVE PROTECTION, PER MAINLINE TRAFFIC DIRECTION, PROVIDE A UNIFORMED LEO AND OFFICIAL PATROL CAR IN ADVANCE OF: THE FIRST ACTIVE WORK AREA THAT DRIVERS WILL ENCOUNTER; OR THE ACTIVE WORK AREA LATERALLY CLOSEST TO THE OPEN TRAVELED LANE; OR OTHER LOCATION AS APPROVED BY THE ENGINEER.

THE UNIFORMED LEO AND OFFICIAL PATROL CAR MAY RELOCATE AMONG THE LISTED LOCATIONS AS APPROPRIATE AS THE OPERATIONS PROCEED IN THE LOCALIZED QUALIFYING WORK AREAS.

IN GENERAL, LEOS SHOULD BE POSITIONED IN ADVANCE OF AND ON THE SAME SIDE AS THE LANE RESTRICTION OR AT THE POINT OF ROAD CLOSURE, AND TO MANUALLY CONTROL TRAFFIC MOVEMENTS THROUGH SIGNALIZED INTERSECTIONS IN WORK ZONES.

LEOS SHOULD NOT FORGO THEIR TRAFFIC CONTROL RESPONSIBILITIES TO APPREHEND MOTORISTS FOR ROUTINE TRAFFIC VIOLATIONS. HOWEVER, IF A MOTORIST'S ACTIONS ARE CONSIDERED TO BE RECKLESS, THEN PURSUIT OF THE MOTORIST IS APPROPRIATE. THE LEOS WORK AT THE DIRECTION OF THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR SECURING THE SERVICES OF THE LEOS WITH THE APPROPRIATE AGENCIES AND COMMUNICATING THE INTENTIONS OF THE PLANS WITH RESPECT TO DUTIES OF THE LEOS. THE ENGINEER SHALL HAVE FINAL CONTROL OVER THE LEOS' DUTIES AND PLACEMENT, AND WILL RESOLVE ANY ISSUES THAT MAY ARISE BETWEEN THE TWO PARTIES. ENSURE PROVIDED LEOS HAVE BEEN TRAINED APPROPRIATE TO THE JOB DECISIONS THEY ARE REQUIRED TO MAKE WHILE ON THE PROJECT, IN ACCORDANCE WITH C&MS 614.03. THE LEO SHALL REPORT IN TO THE CONTRACTOR PRIOR TO THE START OF THE SHIFT, IN ORDER TO RECEIVE INSTRUCTIONS REGARDING SPECIFIC WORK ASSIGNMENTS DURING HIS/HER SHIFT. THE LEO IS EXPECTED TO STAY AT THE PROJECT SITE FOR THE ENTIRE DURATION OF HIS/HER SHIFT. THE LEO SHALL REPORT TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT. SHOULD IT BE NECESSARY TO LEAVE THE PROJECT SITE, THE LEO SHALL NOTIFY THE ENGINEER. THE CONTRACTOR SHALL PROVIDE THE LEO WITH A TWO-WAY COMMUNICATION DEVICE THAT SHALL BE RETURNED TO THE CONTRACTOR AT THE END OF HIS/HER SHIFT. LEOS (WITH PATROL CAR) REQUIRED BY THE TRAFFIC MAINTENANCE TASKS ABOVE SHALL BE PAID FOR ON A UNIT PRICE (HOURLY) BASIS UNDER ITEM 614, LAW ENFORCEMENT OFFICER (WITH PATROL CAR) FOR ASSISTANCE. THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN CARRIED TO THE GENERAL SUMMARY.

ITEM 614, LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE 100 HOURS. THE HOURS PAID SHALL INCLUDE ANY MINIMUM SHOW-UP TIME REQUIRED BY THE LAW ENFORCEMENT AGENCY INVOLVED. ANY ADDITIONAL COSTS (ADMINISTRATIVE OR OTHERWISE) INCURRED BY THE CONTRACTOR TO OBTAIN THE SERVICES OF AN LEO ARE INCLUDED WITH THE BID UNIT PRICE FOR ITEM 614, LAW ENFORCEMENT OFFICER WITH PATROL CAR FOR ASSISTANCE.

ACCESS TO PRIVATE PROPERTY

ACCESS TO DRIVES SHALL BE MAINTAINED VIA EXISTING PAVEMENT, TEMPORARY PAVEMENT IN THE EVENT THAT A DRIVE CANNOT BE MAINTAINED AND A CLOSURE IS NEEDED THE CONTRACTOR WILL COORDINATE WITH THE PROPERTY OWNER TO MINIMIZE THE IMPACT TO THE OWNER. COMMERCIAL PROPERTY WITH MULTIPLE DRIVES MAY HAVE ONE DRIVE CLOSED WHEN WORKING IN THE AREA OF THE DRIVE. COMMERCIAL PROPERTY WITH ONLY ONE DRIVEWAY OR DRIVEWAYS WITH ONE DIRECTION TRAFFIC USE WILL BE CONSTRUCTED PART WIDTH. THE CONTRACTOR WILL COORDINATE WITH THE PROPERTY OWNER TO MINIMIZE THE IMPACT TO THE OWNER. MAINTAIN ACCESS TO RESIDENTIAL PROPERTIES AT ALL TIMES. WHEN A RESIDENTIAL DRIVE IS CLOSED FOR CONSTRUCTION, MAINTAIN ALTERNATE ACCESS TO THE PROPERTY. IT MAY BE REQUIRED FOR THE CONTRACTOR TO MAINTAIN ONE PASSABLE LANE WITHIN A CLOSURE IN ORDER FOR VEHICLES TO ACCESS RESIDENCY WITH A VEHICLE. UNLESS CALLED OUT IN THE PLANS THE CONTRACTOR WILL COORDINATE ANY CLOSURES WITH PROPERTY OWNERS AND BE RESPONSIBLE FOR ANY AND ALL PROPERTY USE AGREEMENTS FOR ALTERNATE ACCESS. SUCCESSFULLY NOTIFY THE OCCUPANTS/OWNERS OF COMMERCIAL OR RESIDENTIAL DRIVES TO BE CLOSED AND COORDINATE THE CLOSURE AT LEAST 48 HOURS BEFORE THE CLOSURE BEGINS (SIMPLY LEAVING A WRITTEN NOTICE OR PHONE MESSAGE IS NOT SUFFICIENT). COORDINATE ALTERNATE ACCESS TO RESIDENTIAL PROPERTIES WITH THE OWNER/OCCUPANT. PAYMENT FOR ALL LABOR, EQUIPMENT, LAW ENFORCEMENT OFFICERS AND MATERIALS SHALL BE INCLUDED IN THE LUMP SUM CONTRACT PRICE FOR ITEM 614, MAINTAINING TRAFFIC UNLESS ITEMIZED SEPARATELY.

ITEM 615 - ROADS FOR MAINTAINING TRAFFIC, AS PER PLAN

IN ADDITION TO THE REQUIREMENTS OF ODOT C&MS 615, THIS ITEM SHALL INCLUDE TEMPORARY DRAINAGE ITEMS, RESTORATION OF ALL SURFACES AND SIGNS DISTURBED BY THE PLACEMENT OF PAVEMENT FOR MAINTAINING TRAFFIC OUTSIDE OF THE PROJECT LIMITS.

TEMPORARY PAVEMENT SECTION

TEMPORARY PAVEMENT SECTION TO BE INSTALLED PER ODOT ITEM 615 FOR S.R. 149/RECO DRIVE. TEMPORARY PAVEMENT SECTION MAY BE SUBSTITUTED AT DIRECTION OF ENGINEER.

ITEM 614 - WORK ZONE IMPACT ATTENUATOR FOR 24" WIDE HAZARDS (UNIDIRECTIONAL OR BIDIRECTIONAL)

THIS ITEM SHALL CONSIST OF FURNISHING AND INSTALLING A NON-GATING IMPACT ATTENUATOR. FURNISH AN IMPACT ATTENUATOR FROM THE OFFICE OF ROADWAY ENGINEERING'S APPROVED LIST FOR WORK ZONE IMPACT ATTENUATORS, FROM THE ROADWAY STANDARDS APPROVED PRODUCTS WEB PAGE. INSTALLATION SHALL BE AT THE LOCATIONS SPECIFIED IN THE PLANS IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS. THE CONTRACTOR SHALL REPAIR OR REPLACE A DAMAGED UNIT WITHIN 24 HOURS OF A DAMAGING IMPACT. WHEN BIDIRECTIONAL DESIGNS ARE SPECIFIED, THE CONTRACTOR SHALL SUPPLY APPROPRIATE TRANSITIONS. WHEN GATING IMPACT ATTENUATORS ARE DESIRED, THE CONTRACTOR SHALL SUBMIT DOCUMENTATION TO THE ENGINEER FOR ACCEPTANCE. THE COST FOR THE ADDITIONAL BARRIER REQUIRED FOR A GATING IMPACT ATTENUATOR SHALL BE INCLUDED IN THE COST OF THE GATING IMPACT ATTENUATOR. PAYMENT FOR THE ABOVE WORK SHALL BE MADE AT THE UNIT PRICE BID AND SHALL INCLUDE ALL LABOR, TOOLS, EQUIPMENT AND MATERIALS NECESSARY TO CONSTRUCT AND MAINTAIN A COMPLETE AND FUNCTIONAL IMPACT ATTENUATOR SYSTEM, INCLUDING ALL RELATED BACKUPS, TRANSITIONS, LEVELING PADS, HARDWARE AND GRADING, NOT SEPARATELY SPECIFIED, AS REQUIRED BY THE MANUFACTURER.

DUST CONTROL

THE CONTRACTOR SHALL FURNISH AND APPLY WATER FOR DUST CONTROL AS DIRECTED BY THE ENGINEER. THE FOLLOWING ESTIMATED QUANTITIES HAVE BEEN INCLUDED FOR DUST CONTROL PURPOSES:

ITEM 616	WATER	9.0 M.GAL.
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CONSTRUCTION SEQUENCE FOR MAINTENANCE OF TRAFFIC (MOT)

PRE-ROADWAY TRAFFIC SIGNAL INSTALLATION:

INSTALL THE PROPOSED TRAFFIC SIGNAL AND ASSOCIATED UNDERGROUND SIGNAL COMPONENTS AT S.R. 149 AND RECO DRIVE. SIGNAL AND SIGNAL COMPONENTS CAN BE INSTALLED CONCURRENTLY WITH ANY OF THE PHASES PRIOR TO FINAL PAVEMENT MARKING OPERATIONS. TRAFFIC SIGNAL HEADS TO REMAIN COVERED AND TRAFFIC SIGNAL REMAIN INOPERABLE UNTIL FINAL PAVEMENT MARKINGS ARE COMPLETE FOR ALL ROADWAY WORK AND ROADWAY IS READY FOR FINAL OPENING.

PRE-PHASE 1A/1B:

INSTALL TEMPORARY PAVEMENT AS SHOWN ON SOUTH SIDE OF RECO DRIVE. TEMPORARY PAVEMENT TO BE INSTALLED IN EXPEDITED FASHION DURING OFF PEAK TRAFFIC HOURS. PROVIDE MAINTENANCE OF TRAFFIC FOR TEMPORARY PAVEMENT INSTALLATION PER OMUTCD STANDARDS/GUIDELINES AND ODOT SCD MT-97.11. EXISTING CONDITION TO BE RESTORED WHEN NO LONGER NEEDED FOR MOT OPERATIONS.

PHASE 1A:

COMPLETE WORK IN NORTHWEST QUADRANT OF BELMONT MORRISTOWN ROAD (S.R. 149) AND RECO DRIVE. SHIFT TRAFFIC AND MODIFY TRAVEL LANES FOR S.R. 149 AND RECO DRIVE AS SHOWN IN PLAN. COMPLETE SOUTH MARATHON DRIVEWAY WORK. NORTH MARATHON DRIVEWAY TO REMAIN OPEN.

PHASE 1B:

RECONFIGURE MOT PHASE 1A AS SHOWN ON PHASE 1B TO REOPEN MARATHON SOUTH DRIVEWAY AND CLOSE NORTH MARATHON DRIVEWAY TO COMPLETE WORK. COMPLETE WORK IN EXPEDITED FASHION DURING OFF PEAK HOURS TO LIMIT DISRUPTION TO TRUCK TRAFFIC AND ADJACENT BUSINESSES.

PRE-PHASE 2:

INSTALL TEMPORARY PAVEMENT AS SHOWN ON EAST SIDE OF S.R. 149. TEMPORARY PAVEMENT TO BE INSTALLED IN EXPEDITED FASHION DURING OFF PEAK TRAFFIC HOURS. PROVIDE MAINTENANCE OF TRAFFIC FOR TEMPORARY PAVEMENT INSTALLATION PER OMUTCD STANDARDS/GUIDELINES AND ODOT SCD MT-97.11. EXISTING CONDITION TO BE RESTORED WHEN NO LONGER NEEDED FOR MOT OPERATIONS.

PHASE 2:

COMPLETE WORK ON WEST SIDE OF BELMONT MORRISTOWN ROAD (S.R. 149) AND SOUTH WEST QUADRANT OF RECO DRIVE. SHIFT TRAFFIC AND MODIFY TRAVEL LANES FOR S.R. 149 AND RECO DRIVE AS SHOWN IN PLAN.

PRE-PHASE 3:

INSTALL TEMPORARY PAVEMENT AS SHOWN ON WEST SIDE OF S.R. 149. TEMPORARY PAVEMENT TO BE INSTALLED IN EXPEDITED FASHION DURING OFF PEAK TRAFFIC HOURS. PROVIDE MAINTENANCE OF TRAFFIC FOR TEMPORARY PAVEMENT INSTALLATION PER OMUTCD STANDARDS/GUIDELINES AND ODOT SCD MT-97.11. EXISTING CONDITION TO BE RESTORED WHEN NO LONGER NEEDED FOR MOT OPERATIONS. CONTRACTOR TO COORDINATE TEMPORARY GUY-WIRES/STABILIZATION FOR UTILITY POLES ADJACENT TO TEMPORARY PAVEMENT PRIOR TO ANY CONSTRUCTION.

PHASE 3:

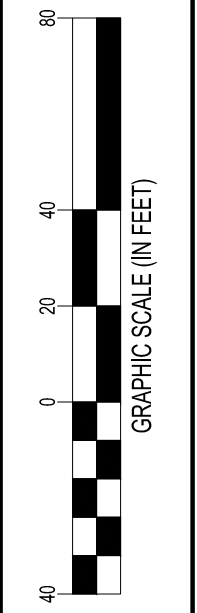
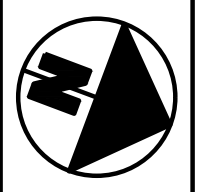
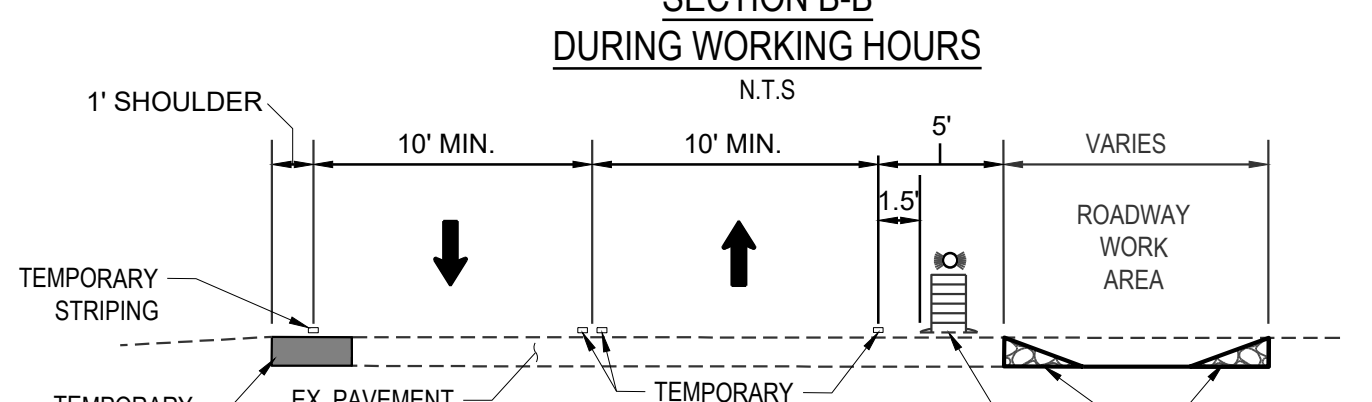
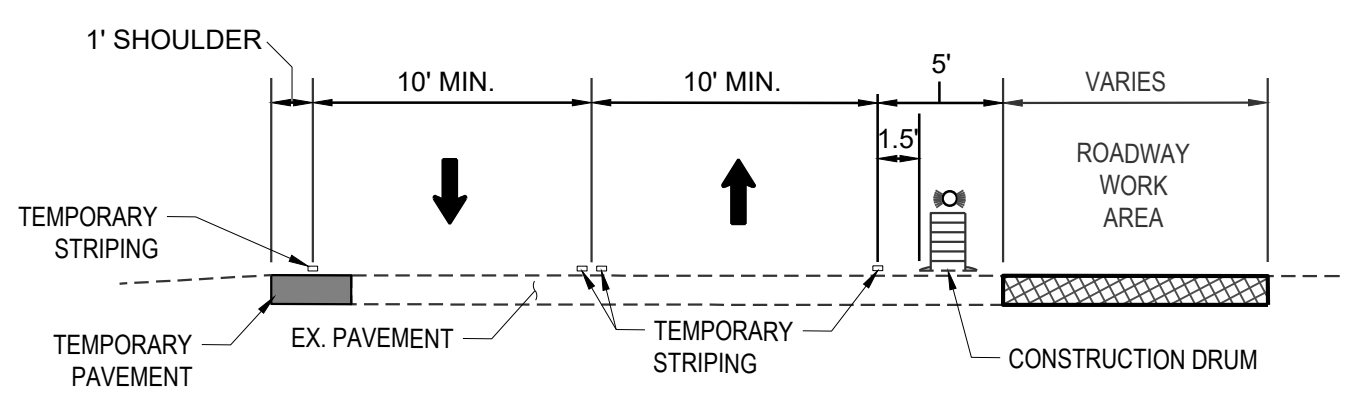
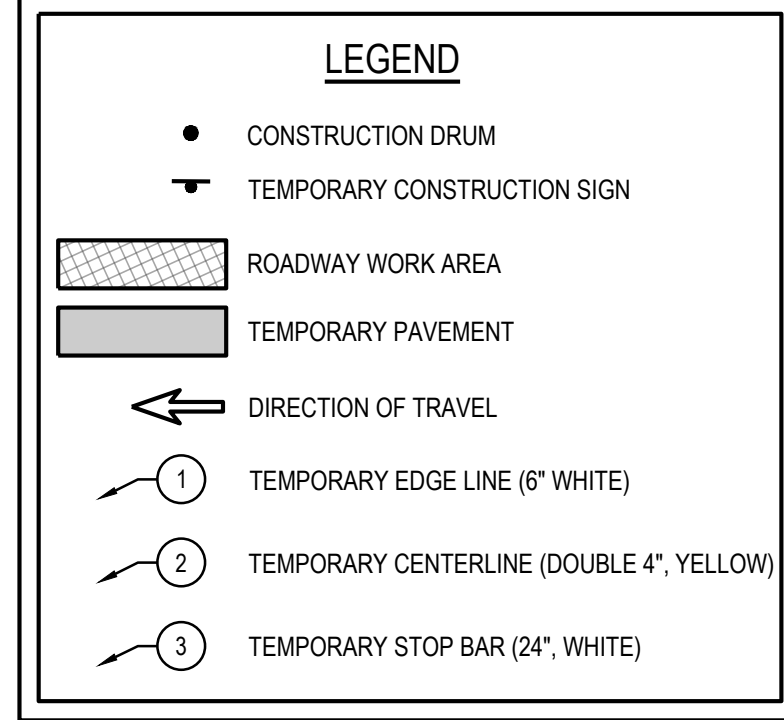
COMPLETE WORK ON EAST SIDE OF BELMONT MORRISTOWN ROAD (S.R. 149). SHIFT TRAFFIC AND MODIFY TRAVEL LANES FOR S.R. 149 AS SHOWN IN PLAN.

MAINTENANCE OF TRAFFIC NOTES
S.R. 149

BEL-149-23.44



- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
 - EXISTING PAVEMENT MARKINGS THAT CONFLICT WITH TEMPORARY TRAFFIC CONTROL MARKINGS OR ARE LABELED TO BE REMOVED SHALL BE COVERED WITH REMOVABLE BLACKOUT TAPE (CMS 740.06, TYPE I).
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE (CMS 740.06, TYPE I)
 - TEMPORARILY BAG/COVER EXISTING SIGNS IN WORK ZONE WHEN THEY CONFLICT WITH INTENDED MAINTENANCE OF TRAFFIC PLAN.
 - STEADY BURNING TYPE A WARNING LIGHTS SHOWN ON THE ADVANCE WARNING SIGNS ARE REQUIRED WHENEVER A NIGHT LANE CLOSURE IS NECESSARY.
 - PROVISIONS SHALL BE MADE TO STABILIZE THE DRUMS TO PREVENT THEM FROM BLOWING OVER.
 - ALL MATERIAL AND EQUIPMENT SHALL BE REMOVED FROM THE CLOSURE AND THE WORK AREA WHEN NO WORK IS BEING DONE.
 - ALL TAPERS AND WORK ZONE TRAFFIC CONTROL SHALL COMPLY WITH REQUIREMENTS OUTLINED IN THE OMUTCD.



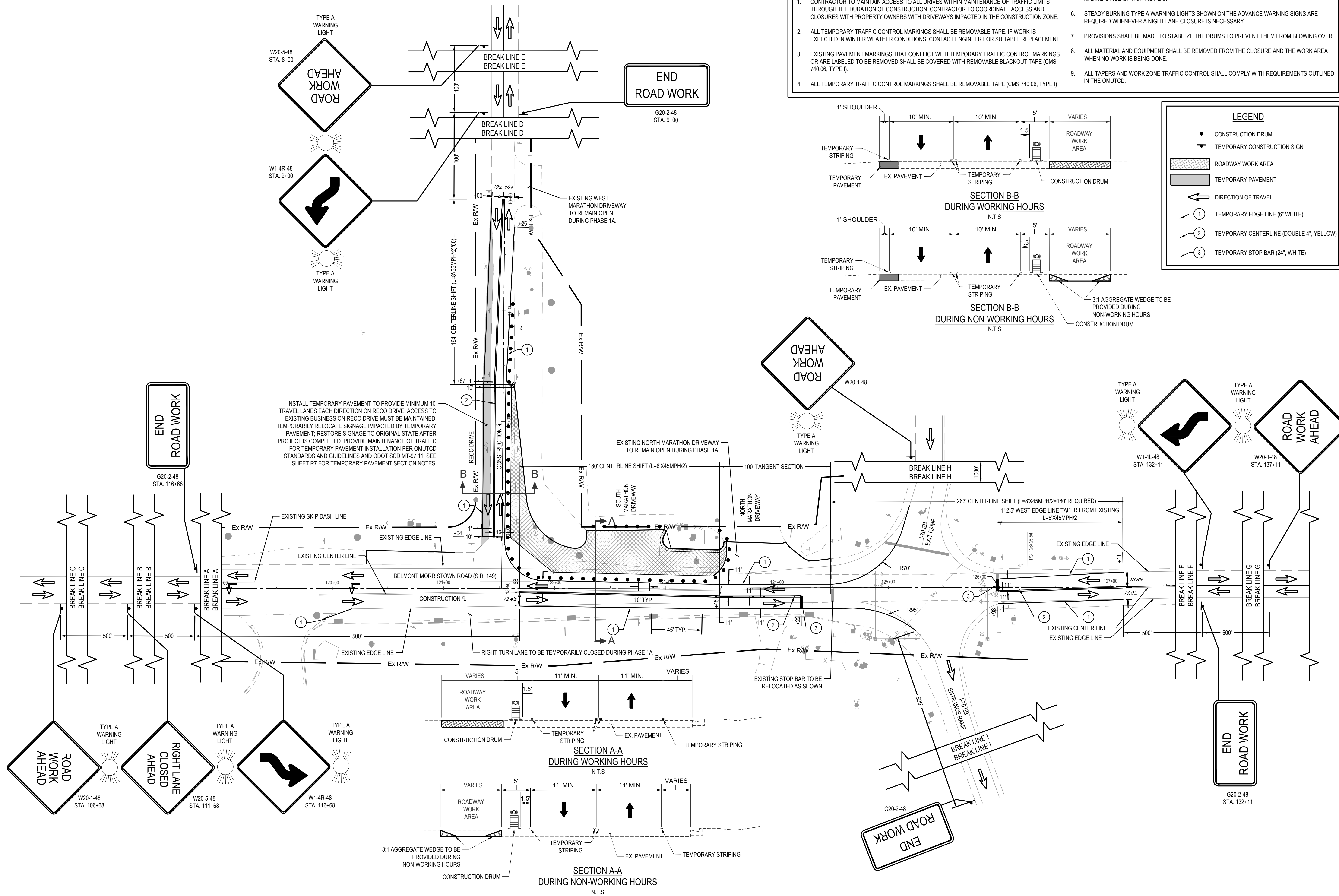
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MAINTENANCE OF TRAFFIC - PHASE 1A
S.R. 149

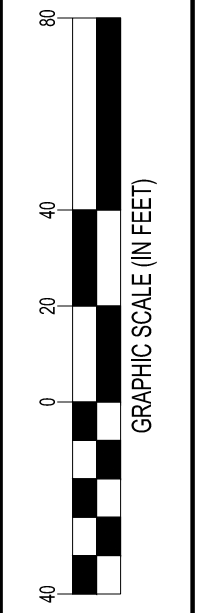
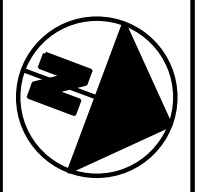
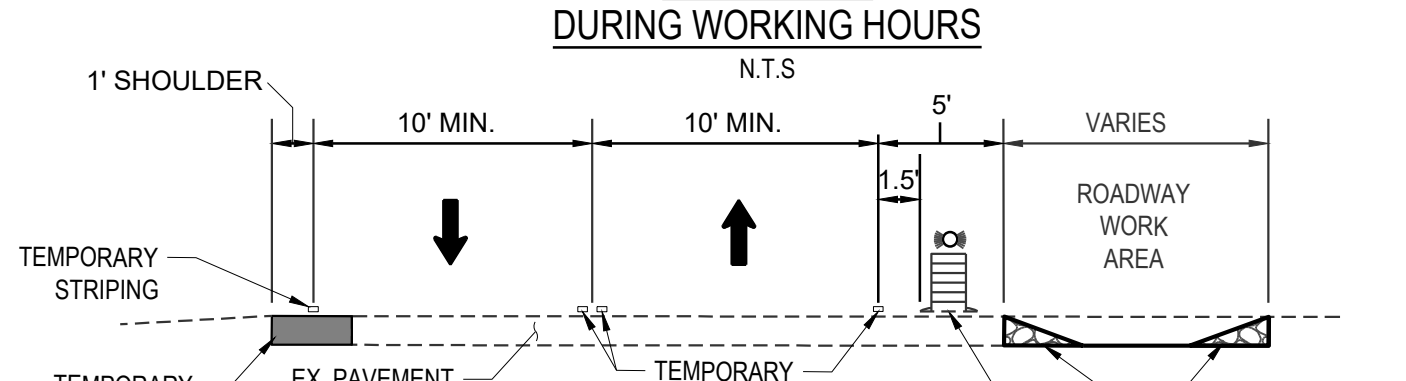
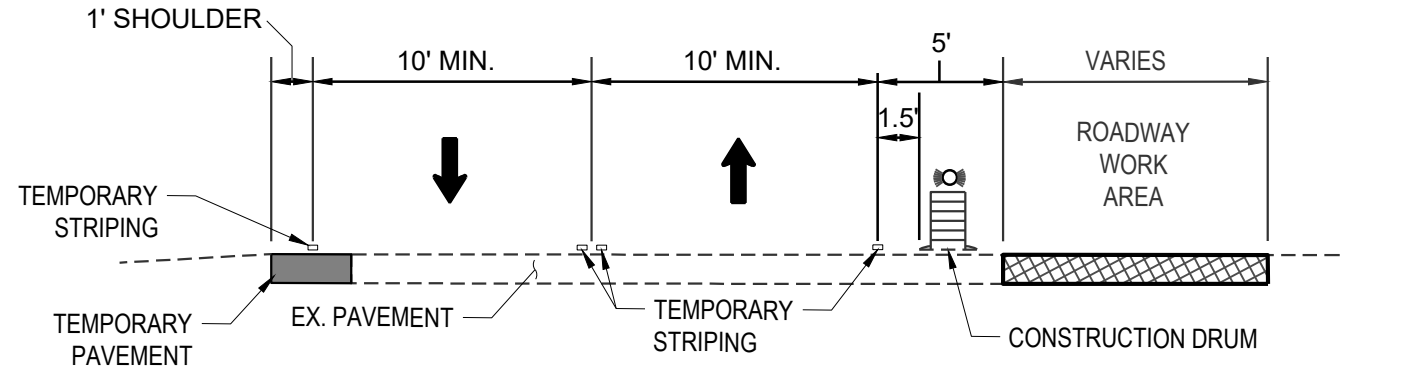
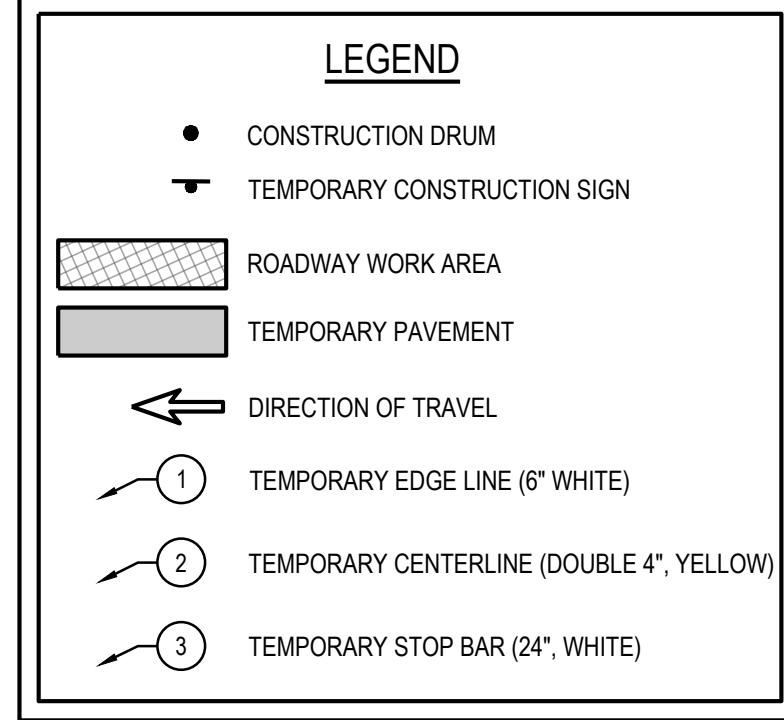
BEL-149-23.44

R8
R31

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- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
 - EXISTING PAVEMENT MARKINGS THAT CONFLICT WITH TEMPORARY TRAFFIC CONTROL MARKINGS OR ARE LABELED TO BE REMOVED SHALL BE COVERED WITH REMOVABLE BLACKOUT TAPE (CMS 740.06, TYPE I).
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 - STEADY BURNING TYPE A WARNING LIGHTS SHOWN ON THE ADVANCE WARNING SIGNS ARE REQUIRED WHENEVER A NIGHT LANE CLOSURE IS NECESSARY.
 - PROVISIONS SHALL BE MADE TO STABILIZE THE DRUMS TO PREVENT THEM FROM BLOWING OVER.
 - ALL MATERIAL AND EQUIPMENT SHALL BE REMOVED FROM THE CLOSURE AND THE WORK AREA WHEN NO WORK IS BEING DONE.
 - ALL TAPERS AND WORK ZONE TRAFFIC CONTROL SHALL COMPLY WITH REQUIREMENTS OUTLINED IN THE OMUTCD.



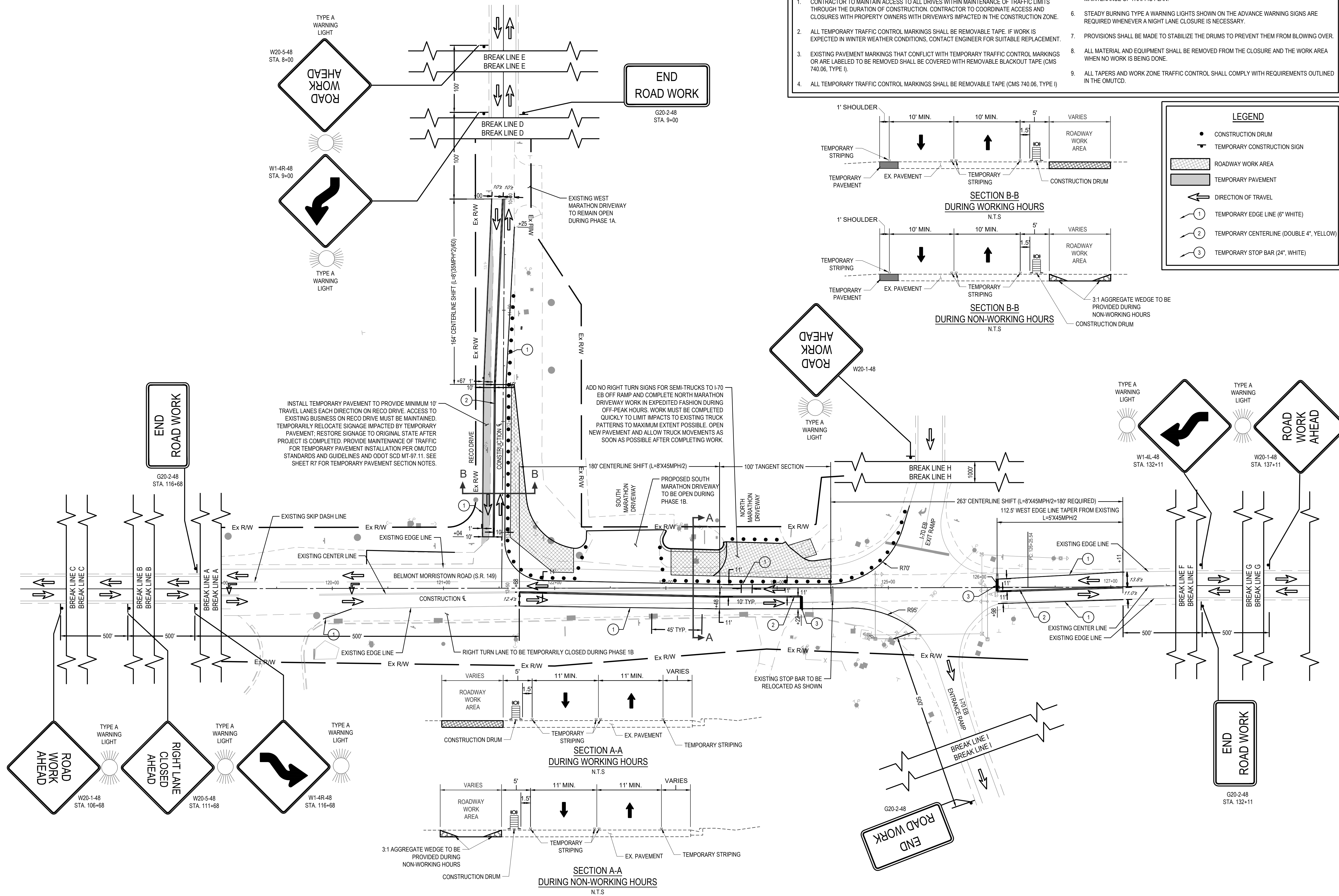
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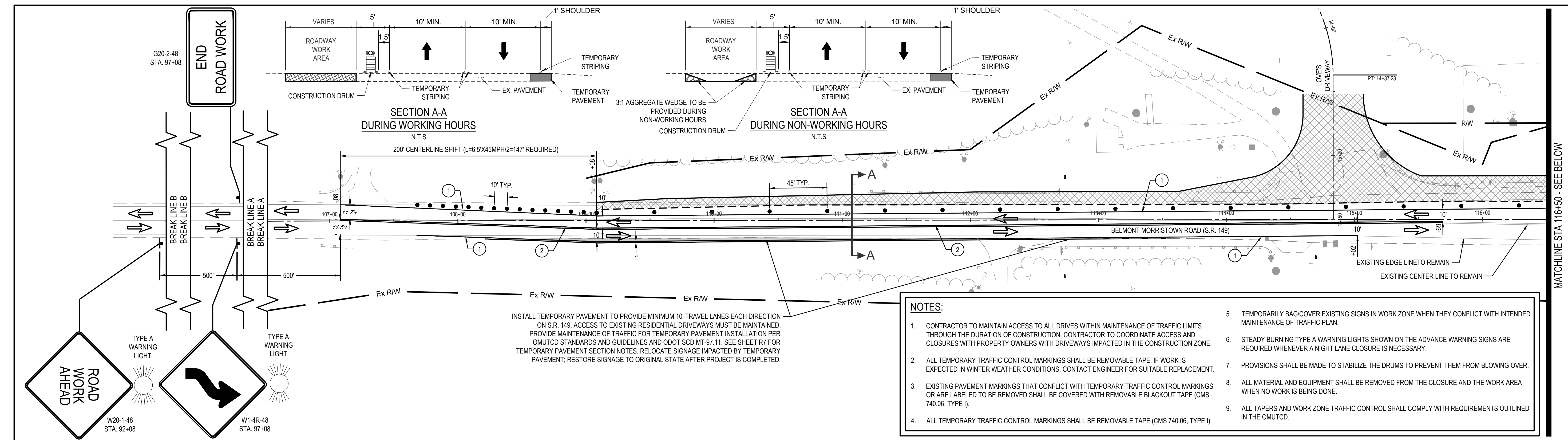
MAINTENANCE OF TRAFFIC - PHASE 1B
S.R. 149

BEL-149-23.44

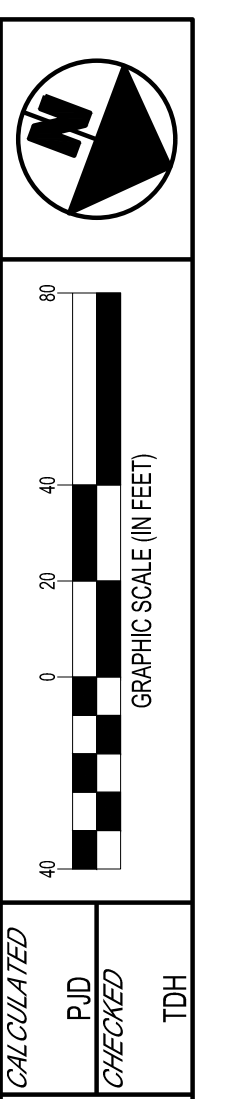
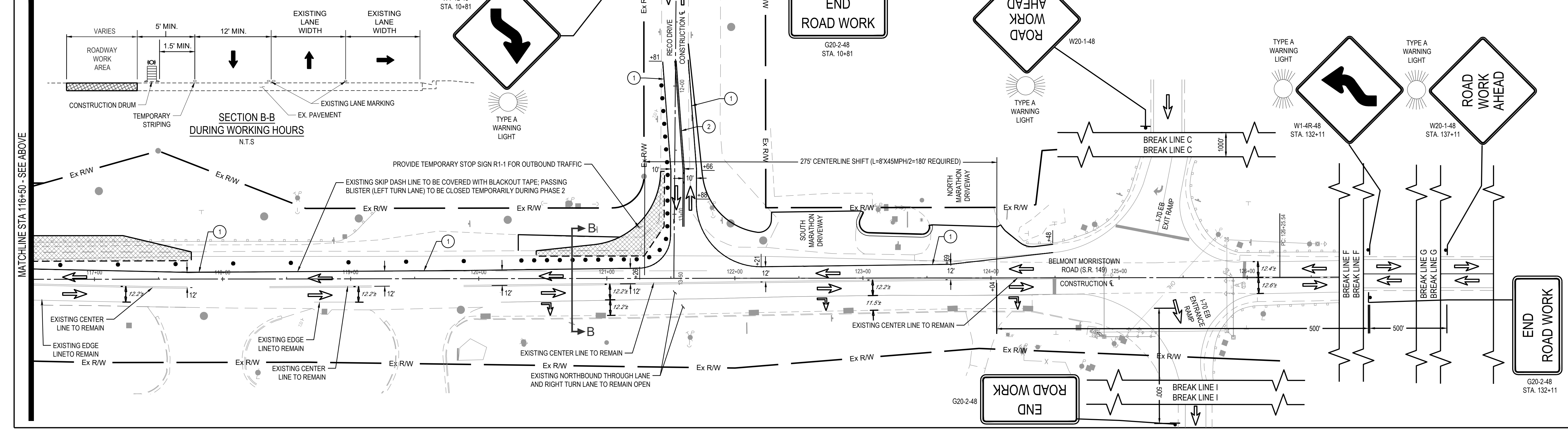
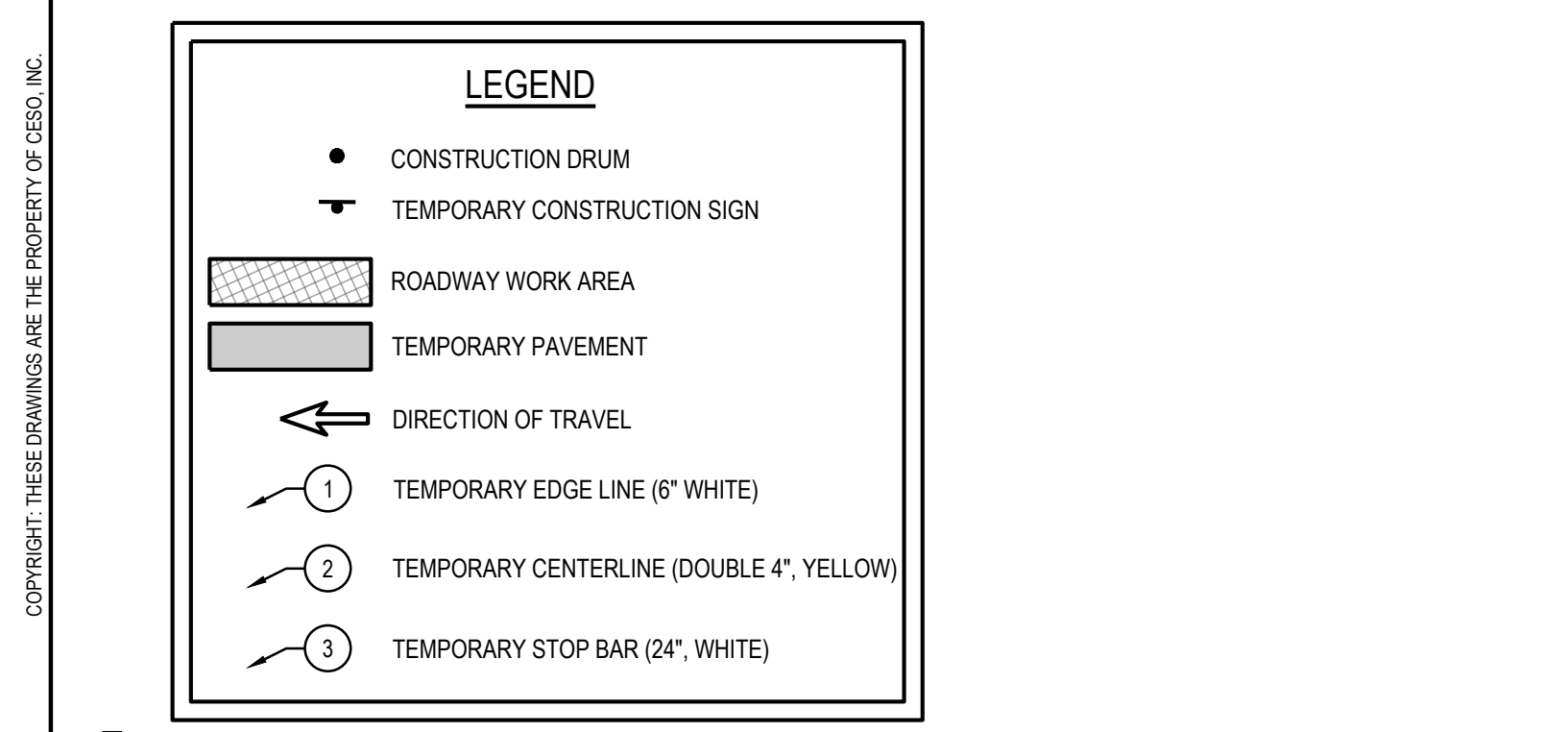
R9
R31

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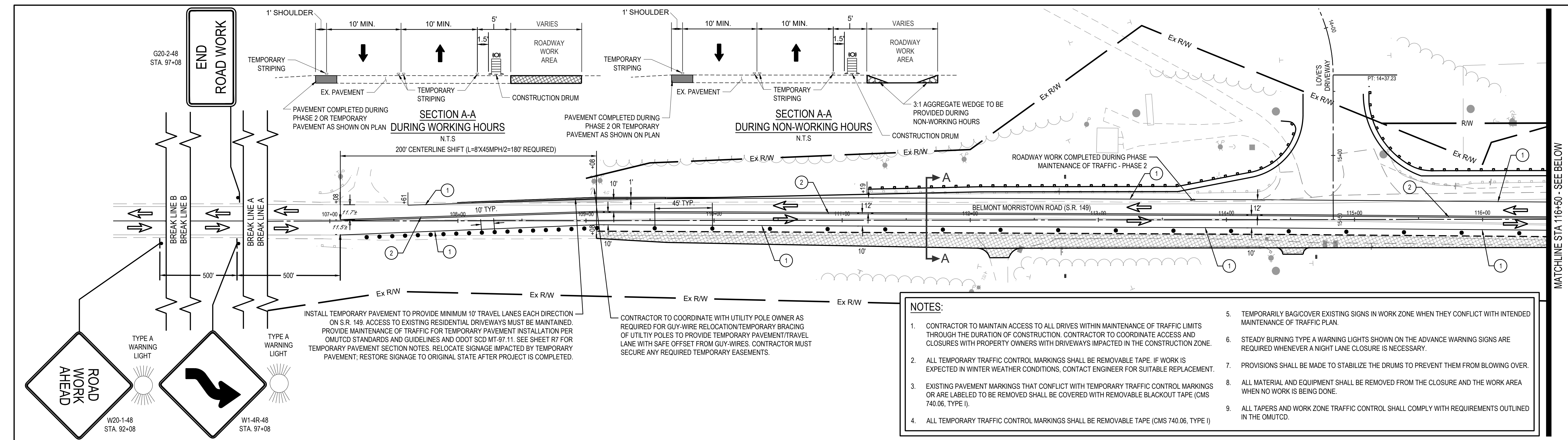


- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
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 - STEADY BURNING TYPE A WARNING LIGHTS SHOWN ON THE ADVANCE WARNING SIGNS ARE REQUIRED WHENEVER A NIGHT LANE CLOSURE IS NECESSARY.
 - PROVISIONS SHALL BE MADE TO STABILIZE THE DRUMS TO PREVENT THEM FROM BLOWING OVER.
 - ALL MATERIAL AND EQUIPMENT SHALL BE REMOVED FROM THE CLOSURE AND THE WORK AREA WHEN NO WORK IS BEING DONE.
 - ALL TAPERS AND WORK ZONE TRAFFIC CONTROL SHALL COMPLY WITH REQUIREMENTS OUTLINED IN THE ODOTCD.



MAINTENANCE OF TRAFFIC - PHASE 2
S.R. 149

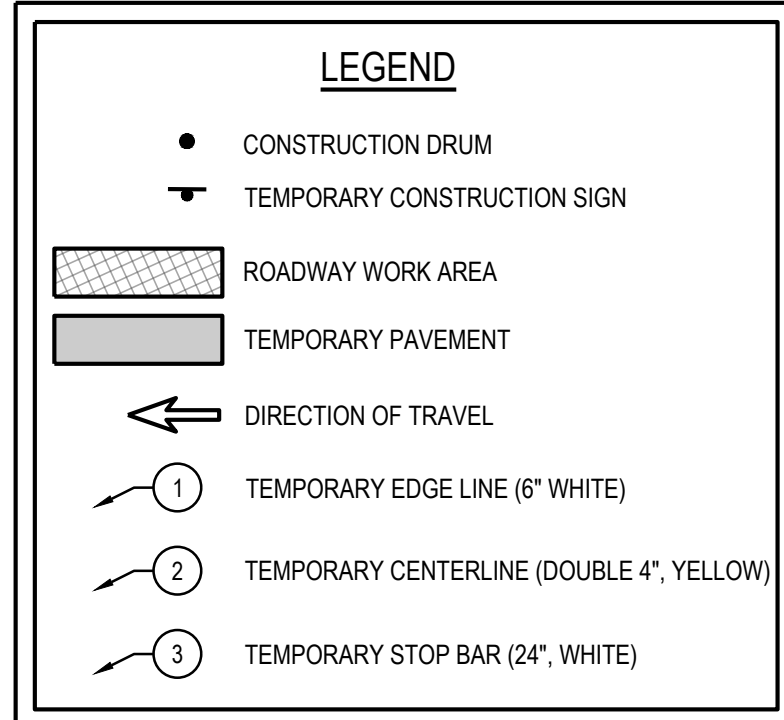
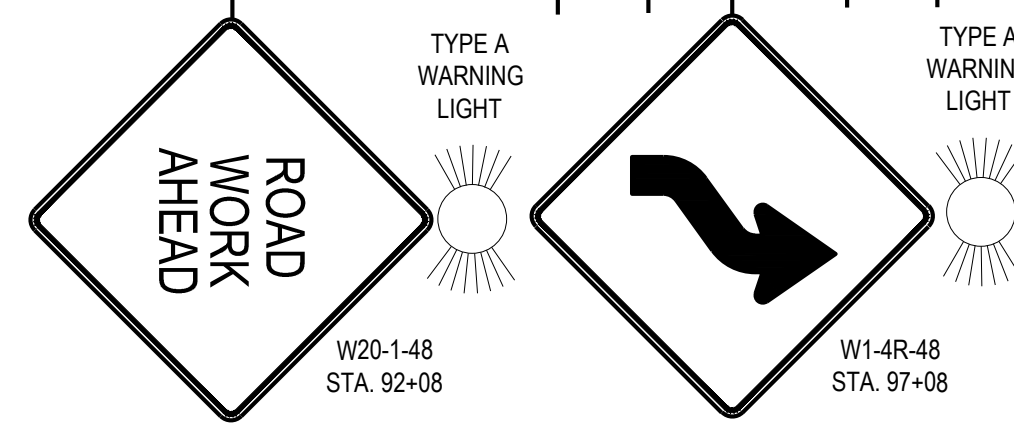
BEL-149-23.44
R10
R31



- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
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 - PROVISIONS SHALL BE MADE TO STABILIZE THE DRUMS TO PREVENT THEM FROM BLOWING OVER.
 - ALL MATERIAL AND EQUIPMENT SHALL BE REMOVED FROM THE CLOSURE AND THE WORK AREA WHEN NO WORK IS BEING DONE.
 - ALL TAPERS AND WORK ZONE TRAFFIC CONTROL SHALL COMPLY WITH REQUIREMENTS OUTLINED IN THE OMTCD.

INSTALL TEMPORARY PAVEMENT TO PROVIDE MINIMUM 10' TRAVEL LANES EACH DIRECTION ON S.R. 149. ACCESS TO EXISTING RESIDENTIAL DRIVEWAYS MUST BE MAINTAINED. PROVIDE MAINTENANCE OF TRAFFIC FOR TEMPORARY PAVEMENT INSTALLATION PER OMTCD STANDARDS AND GUIDELINES AND ODOT SCD MT-97.11. SEE SHEET R7 FOR TEMPORARY PAVEMENT SECTION NOTES. RELOCATE SIGNAGE IMPACTED BY TEMPORARY PAVEMENT; RESTORE SIGNAGE TO ORIGINAL STATE AFTER PROJECT IS COMPLETED.

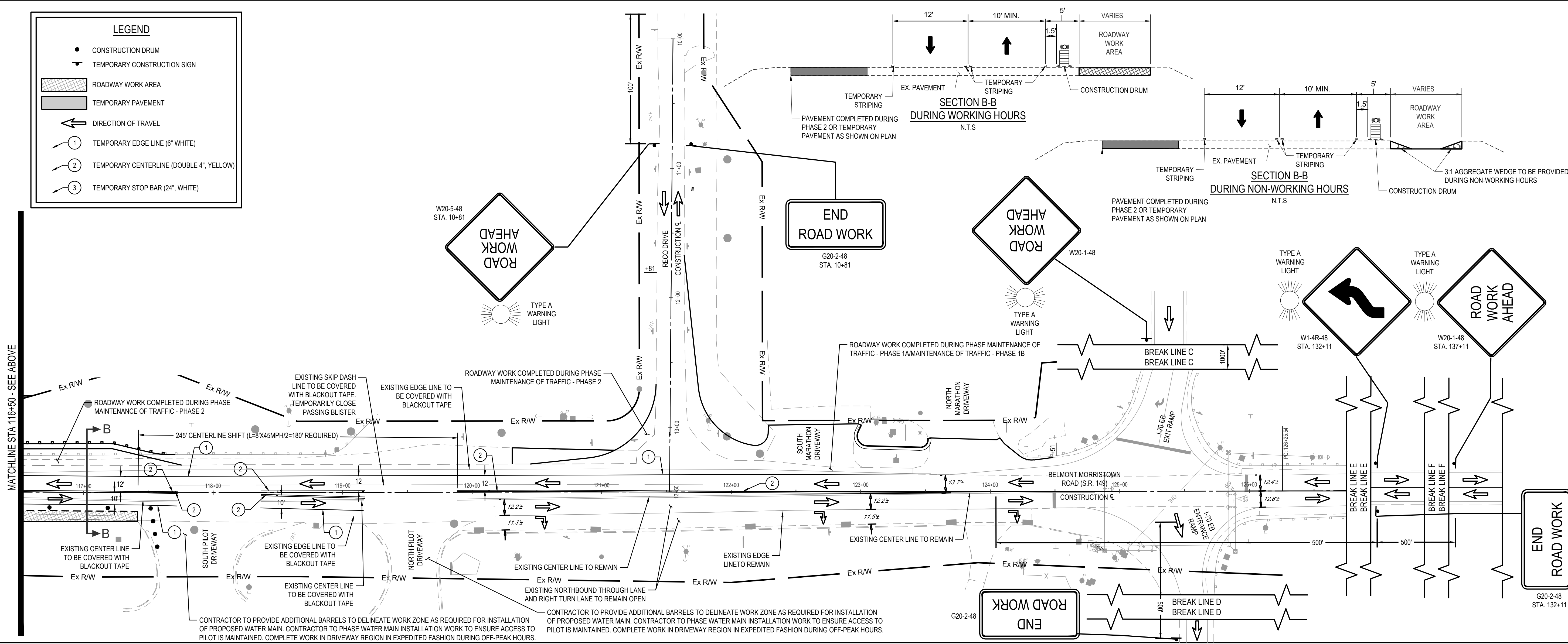
CONTRACTOR TO COORDINATE WITH UTILITY POLE OWNER AS REQUIRED FOR GUY-WIRE RELOCATION/TEMPORARY BRACING OF UTILITY POLES TO PROVIDE TEMPORARY PAVEMENT/TRAVEL LANE WITH SAFE OFFSET FROM GUY-WIRES. CONTRACTOR MUST SECURE ANY REQUIRED TEMPORARY EASEMENTS.



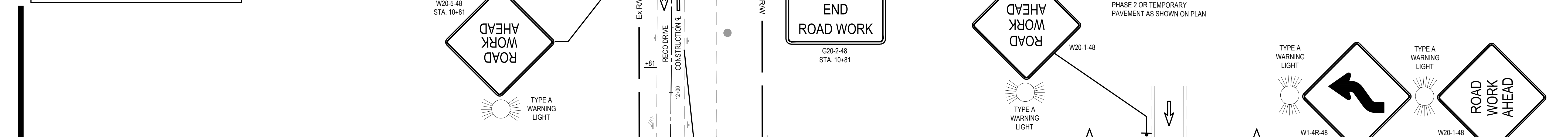
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MATCHLINE STA 116+50 - SEE ABOVE

MATCHLINE STA 116+50 - SEE BELOW

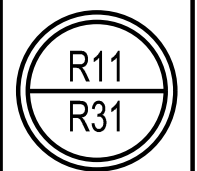


CONTRACTOR TO PROVIDE ADDITIONAL BARRELS TO DELINEATE WORK ZONE AS REQUIRED FOR INSTALLATION OF PROPOSED WATER MAIN. CONTRACTOR TO PHASE WATER MAIN INSTALLATION WORK TO ENSURE ACCESS TO PILOT IS MAINTAINED. COMPLETE WORK IN DRIVEWAY REGION IN EXPEDITED FASHION DURING OFF-PEAK HOURS.



MAINTENANCE OF TRAFFIC - PHASE 3
S.R. 149

BEL-149-23.44



SITE DESCRIPTION

CONSTRUCTION TO WIDEN EAST AND WEST SIDE OF S.R. 149 TO PROVIDE 175' NB LEFT TURN LANE INTO LOVE'S DRIVEWAY, 175' SB RIGHT-TURN LANE INTO LOVE'S DRIVEWAY, 164' TWLTL BETWEEN SOUTH PILOT DRIVEWAY & NORTH PILOT DRIVEWAY, 125' NB LEFT TURN LANE INTO RECO DRIVE, 200' SB RIGHT TURN LANE INTO RECO DRIVE AND ASSOCIATED LANE RECONFIGURATION TO ACCOMMODATE FOREMENTIONED IMPROVEMENTS. CONSTRUCTION TO WIDEN RECO DRIVE AND INSTALLATION OF TRAFFIC SIGNAL AT S.R. 149 & RECO DRIVE INTERSECTION. DITCH AND SLOPE GRADING AND INSTALLATION OF NECESSARY TRAFFIC CONTROL DEVICES, STORMWATER STRUCTURE AND CONDUIT MODIFICATIONS BETWEEN SOUTH MARATHON DRIVEWAY AND NORTH MARATHON DRIVEWAY.

NOI AND BMP NOTE

NOI AND BMP'S FOR ROADWAY WORK ARE INCLUDED WITH OVERALL SITE SUBMISSION FOR LOVE'S TRAVEL STOP DEVELOPMENT.

EARTH DISTURBED AREAS AND TOTAL AREA OF PROJECT

TOTAL AREA OF PROJECT: 1.95 ACRES
 PROJECT EARTH DISTURBING ACTIVITIES AREA: 1.65 ACRES
 CONTRACTOR EARTH DISTURBING ACTIVITIES AREA: 0.30 ACRES
 NOI EARTH DISTURBING ACTIVITIES AREA: 1.95 ACRES

SITE LOCATION

LAT = N 40° 03' 27"
 LONG = W 81° 03' 09"

REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 4-8-2024 FOR SPECIAL REINFORCED SOIL SLOPE (RSS) DESIGN FROM STA. 112+80 TO STA. 113+33.

REFER TO SITE PLANS FOR SITE GRADING, ROADWAY CONTRACTOR TO COORDINATE GRADING TIE IN AND ON SITE SWPPP MEASURES WITH SITE CONTRACTOR

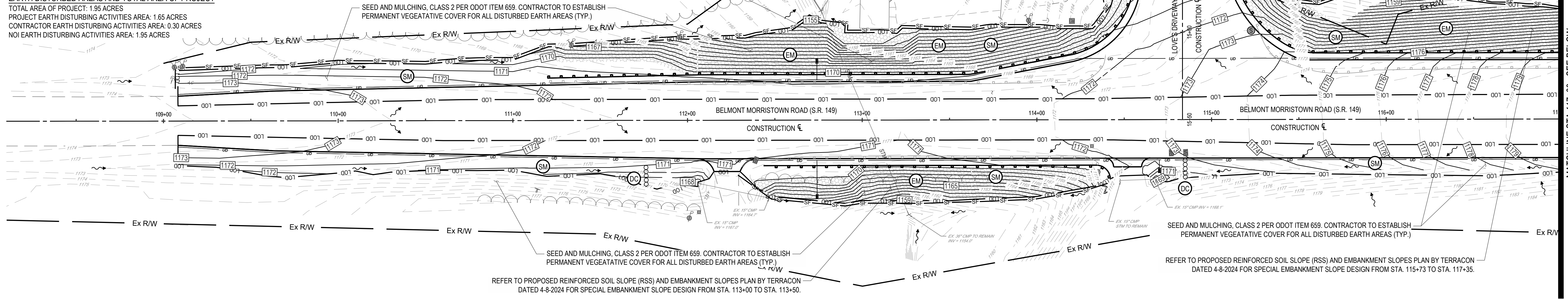
REFER TO SITE PLANS FOR SITE GRADING, ROADWAY CONTRACTOR TO COORDINATE GRADING TIE IN AND ON SITE SWPPP MEASURES WITH SITE CONTRACTOR

SEED AND MULCHING, CLASS 2 PER ODOT ITEM 659. CONTRACTOR TO ESTABLISH PERMANENT VEGETATIVE COVER FOR ALL DISTURBED EARTH AREAS (TYP.)

SEED AND MULCHING, CLASS 2 PER ODOT ITEM 659. CONTRACTOR TO ESTABLISH PERMANENT VEGETATIVE COVER FOR ALL DISTURBED EARTH AREAS (TYP.)

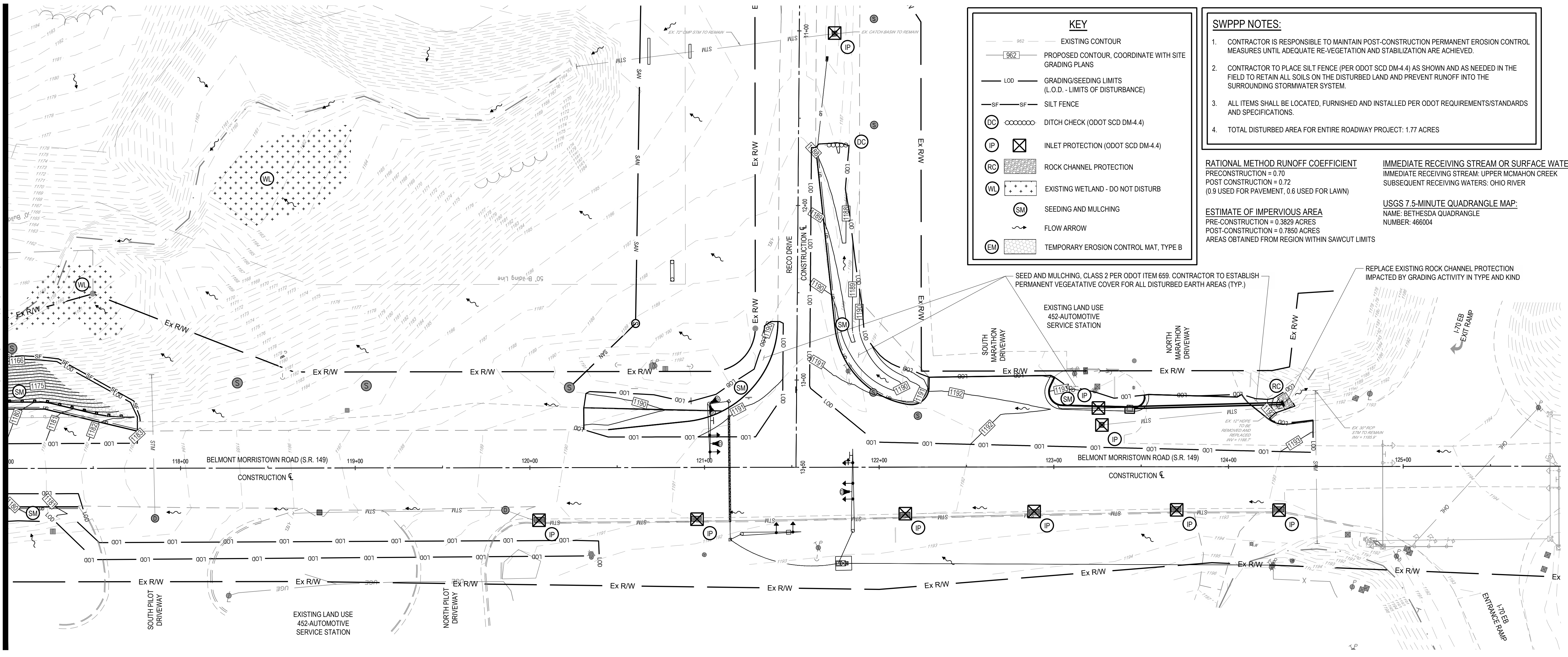
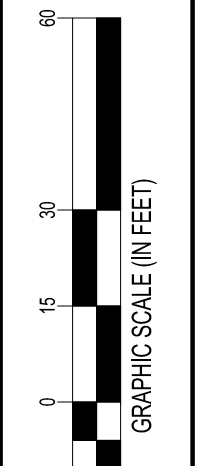
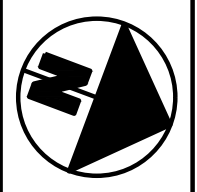
REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 4-8-2024 FOR SPECIAL EMBANKMENT SLOPE DESIGN FROM STA. 113+00 TO STA. 113+50.

REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 4-8-2024 FOR SPECIAL EMBANKMENT SLOPE DESIGN FROM STA. 115+73 TO STA. 117+35.



MATCHLINE STA 117+00 - SEE BELOW

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KEY

- 962 — EXISTING CONTOUR
- 962 — PROPOSED CONTOUR, COORDINATE WITH SITE GRADING PLANS
- LOD — GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
- SF — SF SILT FENCE
- ⊙ DC ⊙ DITCH CHECK (ODOT SCD DM-4.4)
- ⊙ IP ⊙ INLET PROTECTION (ODOT SCD DM-4.4)
- ⊙ RC ⊙ ROCK CHANNEL PROTECTION
- ⊙ WL ⊙ EXISTING WETLAND - DO NOT DISTURB
- ⊙ SM ⊙ SEEDING AND MULCHING
- FLOW ARROW
- ⊙ EM ⊙ TEMPORARY EROSION CONTROL MAT, TYPE B

SWPPP NOTES:

- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- CONTRACTOR TO PLACE SILT FENCE (PER ODOT SCD DM-4.4) AS SHOWN AND AS NEEDED IN THE FIELD TO RETAIN ALL SOILS ON THE DISTURBED LAND AND PREVENT RUNOFF INTO THE SURROUNDING STORMWATER SYSTEM.
- ALL ITEMS SHALL BE LOCATED, FURNISHED AND INSTALLED PER ODOT REQUIREMENTS/STANDARDS AND SPECIFICATIONS.
- TOTAL DISTURBED AREA FOR ENTIRE ROADWAY PROJECT: 1.77 ACRES

RATIONAL METHOD RUNOFF COEFFICIENT
 PRECONSTRUCTION = 0.70
 POST CONSTRUCTION = 0.72
 (0.9 USED FOR PAVEMENT, 0.6 USED FOR LAWN)

IMMEDIATE RECEIVING STREAM OR SURFACE WATER:
 IMMEDIATE RECEIVING STREAM: UPPER MCMAHON CREEK
 SUBSEQUENT RECEIVING WATERS: OHIO RIVER

USGS 7.5-MINUTE QUADRANGLE MAP:
 NAME: BETHESDA QUADRANGLE
 NUMBER: 466004

ESTIMATE OF IMPERVIOUS AREA
 PRE-CONSTRUCTION = 0.3829 ACRES
 POST-CONSTRUCTION = 0.7850 ACRES
 AREAS OBTAINED FROM REGION WITHIN SAWCUT LIMITS

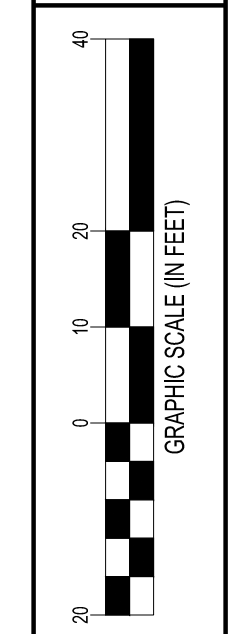
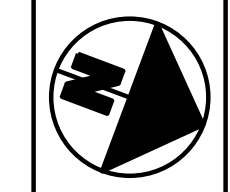
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MATCHLINE STA 117+00 - SEE ABOVE

SITE PLAN
S.R. 149

BEL-149-23.44

R12
R31

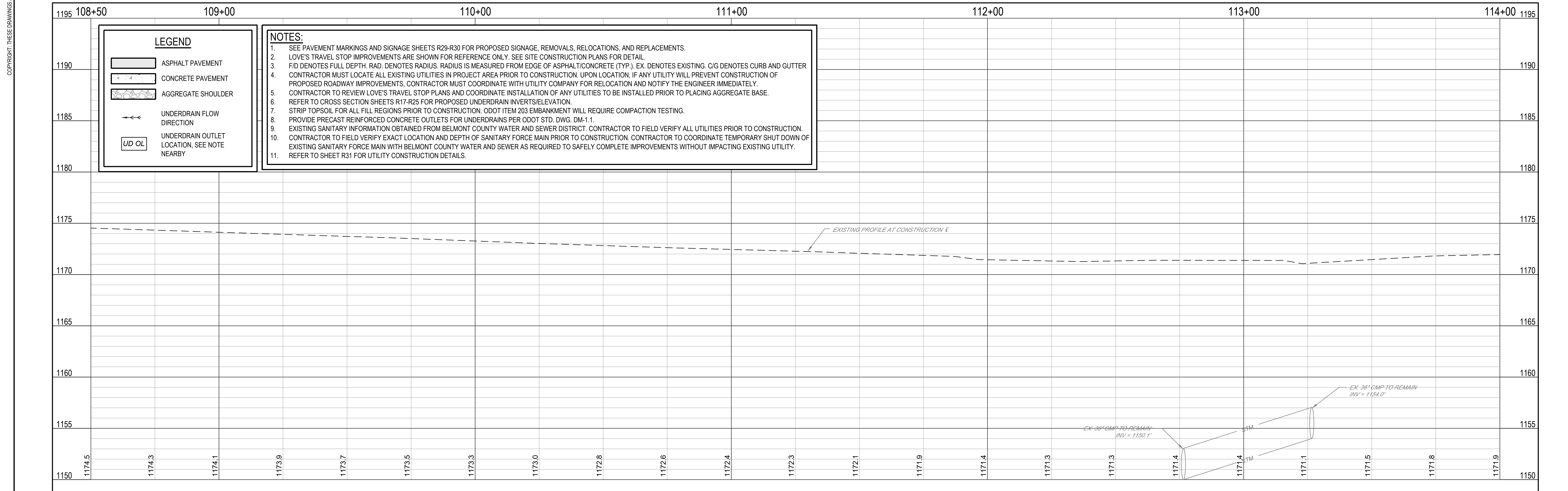
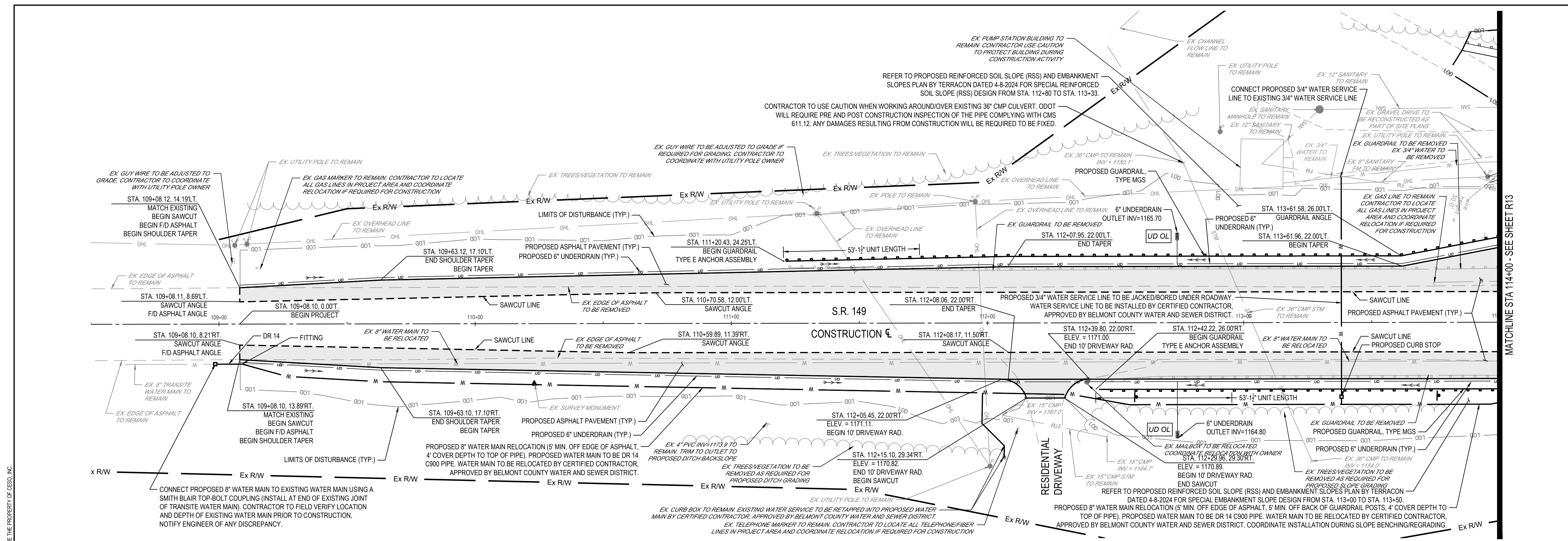


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PLAN & PROFILE - STA. 108+50 TO STA. 114+00
S.R. 149

BEL-149-23.44

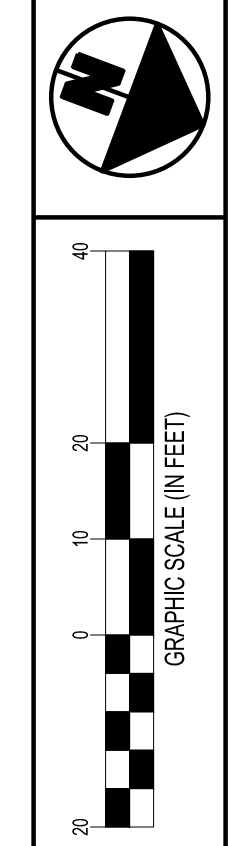
R13
R31



STATE ROUTE 149 PROFILE
Scale: 1" = 20' Horiz.; 1" = 5' Vert.

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MATCHLINE STA 114+00 - SEE SHEET R13

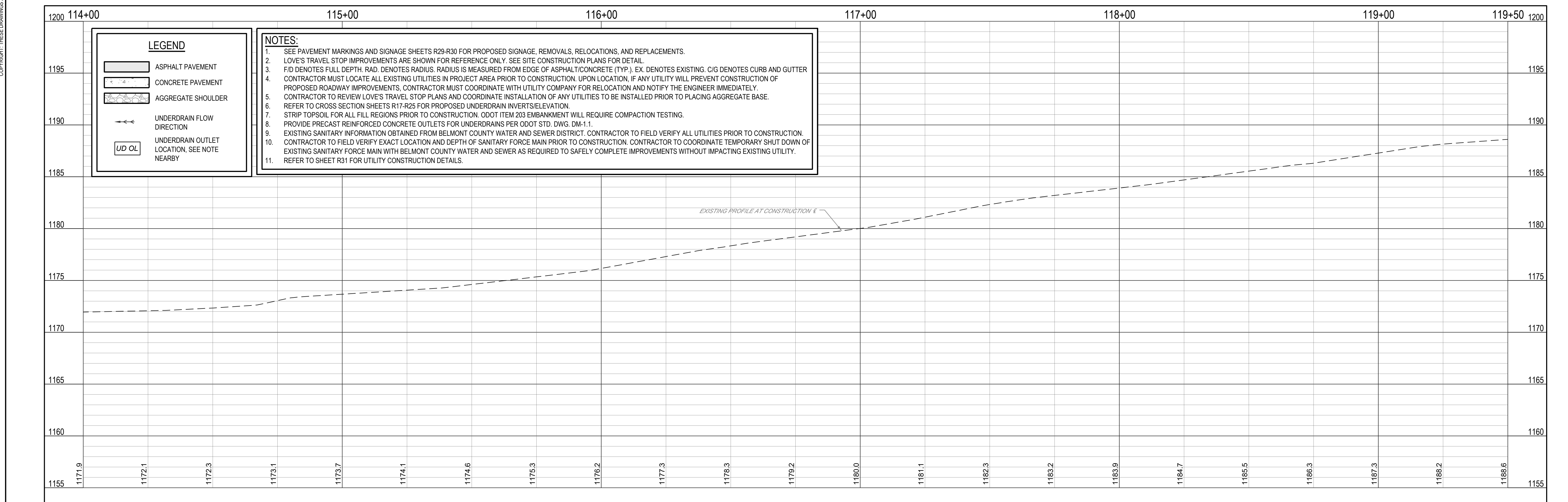
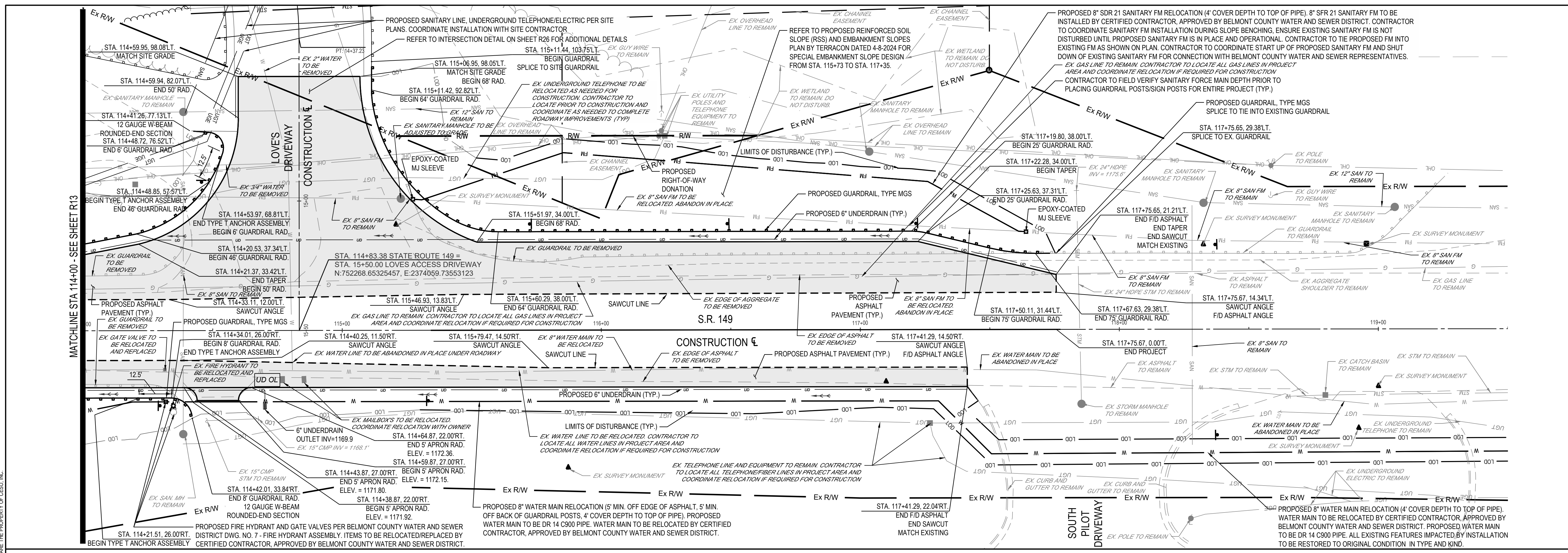


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PLAN & PROFILE - STA. 114+00 TO STA. 119+50
S.R. 149

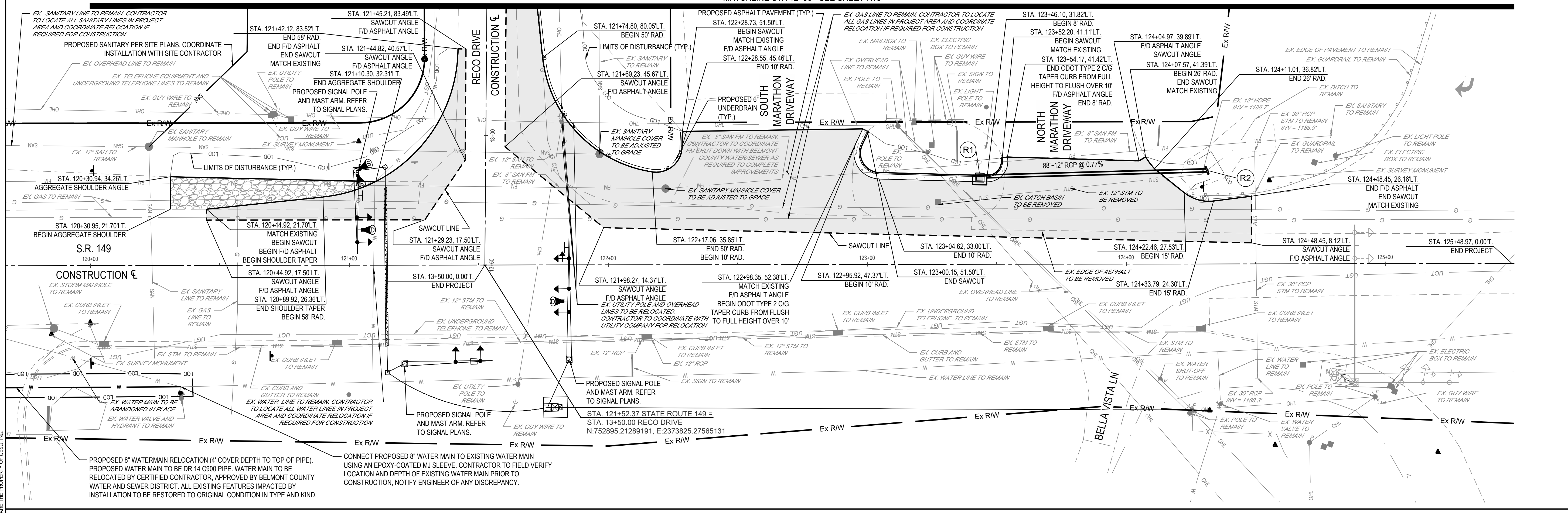
BEL-149-23.44

R14
R31



STATE ROUTE 149 PROFILE
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MATCHLINE STA 12+50 - SEE SHEET R16



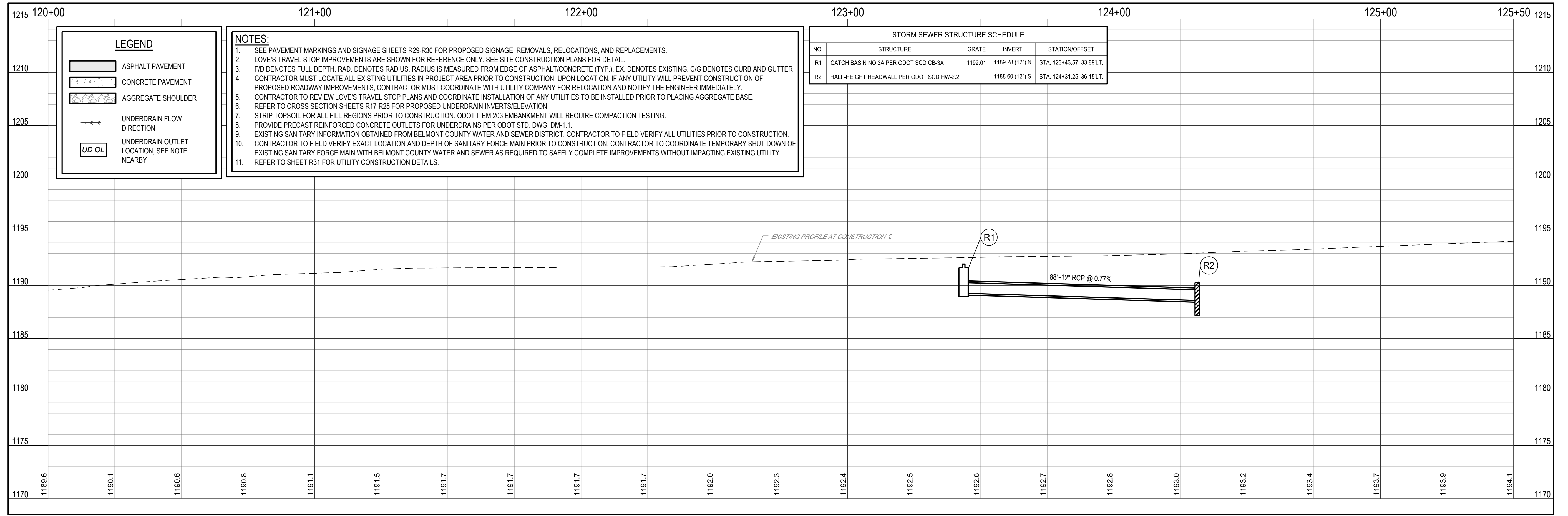
LEGEND

- ASPHALT PAVEMENT
- CONCRETE PAVEMENT
- AGGREGATE SHOULDER
- UNDERDRAIN FLOW DIRECTION
- UNDERDRAIN OUTLET LOCATION. SEE NOTE NEARBY

- NOTES:**
1. SEE PAVEMENT MARKINGS AND SIGNAGE SHEETS R29-R30 FOR PROPOSED SIGNAGE, REMOVALS, RELOCATIONS, AND REPLACEMENTS.
 2. LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
 3. F/D DENOTES FULL DEPTH. RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.). EX. DENOTES EXISTING. C/G DENOTES CURB AND GUTTER. CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.
 4. CONTRACTOR TO REVIEW LOVE'S TRAVEL STOP PLANS AND COORDINATE INSTALLATION OF ANY UTILITIES TO BE INSTALLED PRIOR TO PLACING AGGREGATE BASE.
 5. REFER TO CROSS SECTION SHEETS R17-R25 FOR PROPOSED UNDERDRAIN INVERTS/ELEVATION.
 6. STRIP TOPSOIL FOR ALL FILL REGIONS PRIOR TO CONSTRUCTION. ODOT ITEM 203 EMBANKMENT WILL REQUIRE COMPACTION TESTING.
 7. PROVIDE PRECAST REINFORCED CONCRETE OUTLETS FOR UNDERDRAINS PER ODOT STD. DWG. DM-1.1.
 8. EXISTING SANITARY INFORMATION OBTAINED FROM BELMONT COUNTY WATER AND SEWER DISTRICT. CONTRACTOR TO FIELD VERIFY ALL UTILITIES PRIOR TO CONSTRUCTION.
 9. CONTRACTOR TO FIELD VERIFY EXACT LOCATION AND DEPTH OF SANITARY FORCE MAIN PRIOR TO CONSTRUCTION. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING SANITARY FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER AS REQUIRED TO SAFELY COMPLETE IMPROVEMENTS WITHOUT IMPACTING EXISTING UTILITY.
 10. REFER TO SHEET R31 FOR UTILITY CONSTRUCTION DETAILS.
 - 11.

STORM SEWER STRUCTURE SCHEDULE

NO.	STRUCTURE	GRATE	INVERT	STATION/OFFSET
R1	CATCH BASIN NO.3A PER ODOT SCD CB-3A	1192.01	1189.28 (12\"/>	



STATE ROUTE 149 PROFILE
Scale: 1" = 20' Horiz.; 1" = 5' Vert.

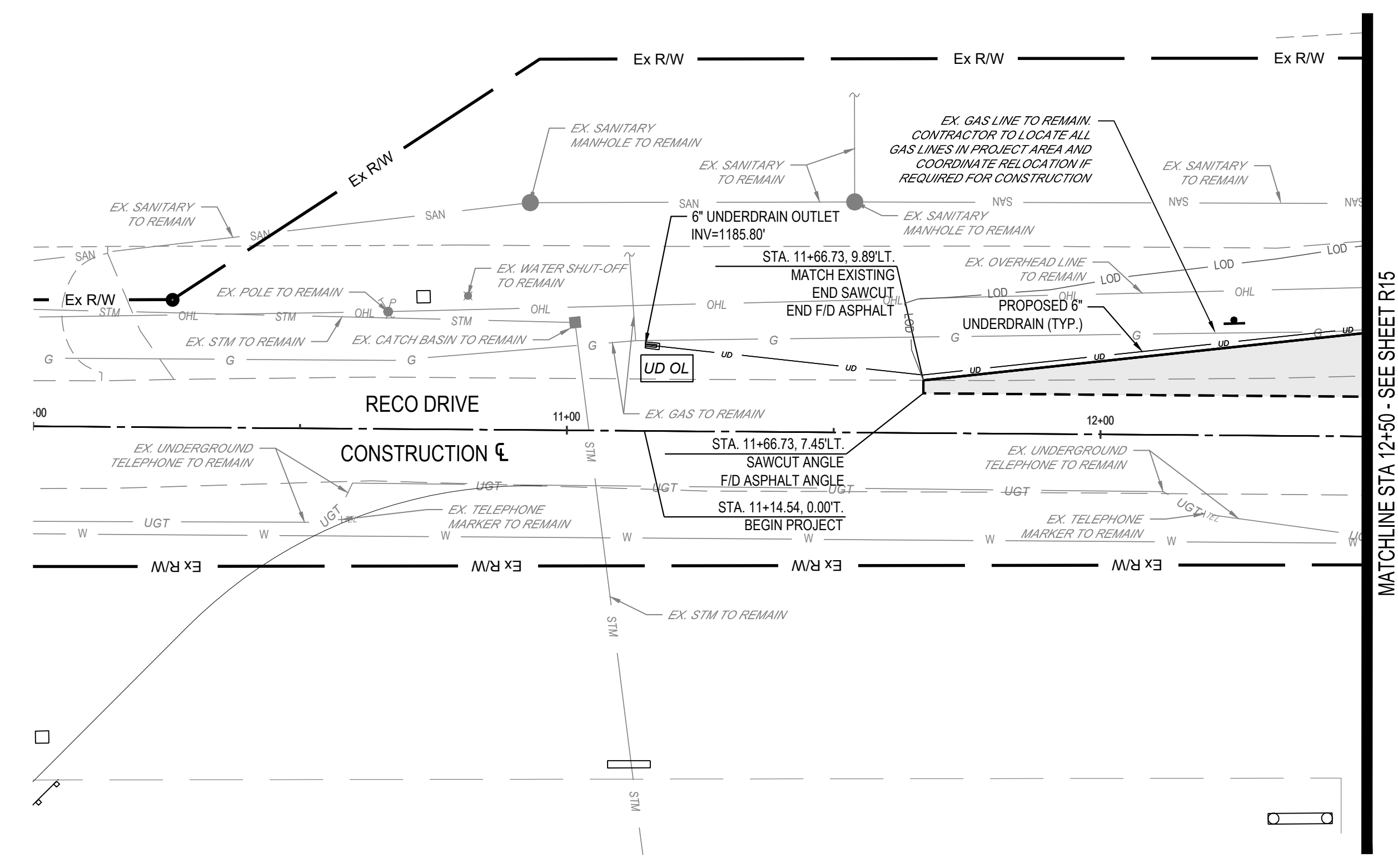
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PLAN & PROFILE - STA. 120+00 TO STA. 125+50
S.R. 149

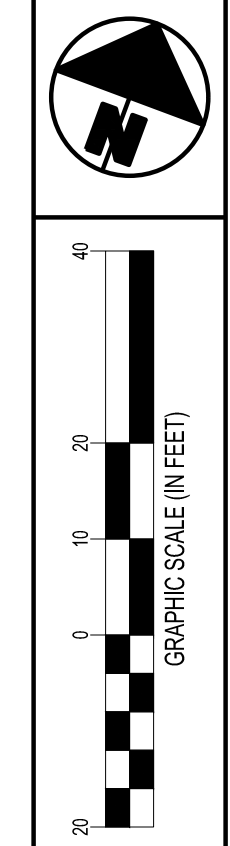
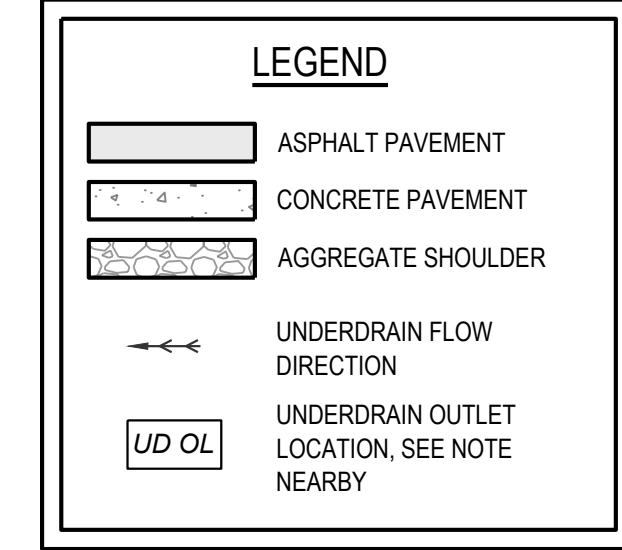
BEL-149-23.44

R15
R31

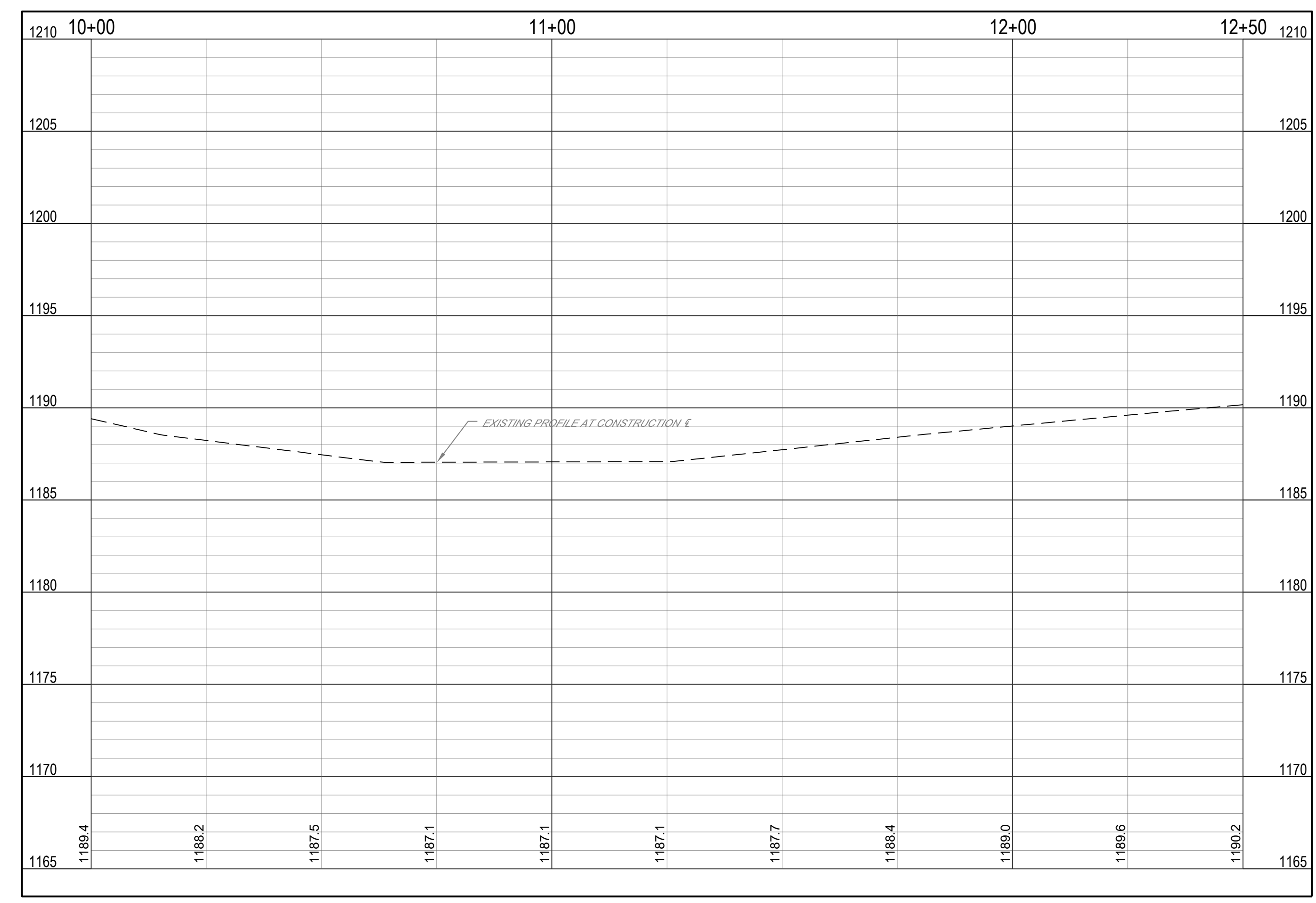
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- NOTES:**
- SEE PAVEMENT MARKINGS AND SIGNAGE SHEETS R29-R30 FOR PROPOSED SIGNAGE, REMOVALS, RELOCATIONS, AND REPLACEMENTS.
 - LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
 - F/D DENOTES FULL DEPTH. RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.). EX. DENOTES EXISTING. C/G DENOTES CURB AND GUTTER.
 - CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.
 - CONTRACTOR TO REVIEW LOVE'S TRAVEL STOP PLANS AND COORDINATE INSTALLATION OF ANY UTILITIES TO BE INSTALLED PRIOR TO PLACING AGGREGATE BASE.
 - REFER TO CROSS SECTION SHEETS R17-R25 FOR PROPOSED UNDERDRAIN INVERTS/ELEVATION.
 - STRIP TOPSOIL FOR ALL FILL REGIONS PRIOR TO CONSTRUCTION. ODOT ITEM 203 EMBANKMENT WILL REQUIRE COMPACTION TESTING.
 - PROVIDE PRECAST REINFORCED CONCRETE OUTLETS FOR UNDERDRAINS PER ODOT STD. DWG. DM-1.1.
 - EXISTING SANITARY INFORMATION OBTAINED FROM BELMONT COUNTY WATER AND SEWER DISTRICT. CONTRACTOR TO FIELD VERIFY ALL UTILITIES PRIOR TO CONSTRUCTION.
 - CONTRACTOR TO FIELD VERIFY EXACT LOCATION AND DEPTH OF SANITARY FORCE MAIN PRIOR TO CONSTRUCTION. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING SANITARY FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER AS REQUIRED TO SAFELY COMPLETE IMPROVEMENTS WITHOUT IMPACTING EXISTING UTILITY.
 - REFER TO SHEET R31 FOR UTILITY CONSTRUCTION DETAILS.



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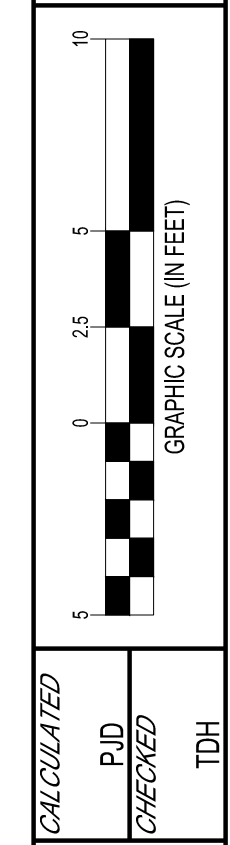
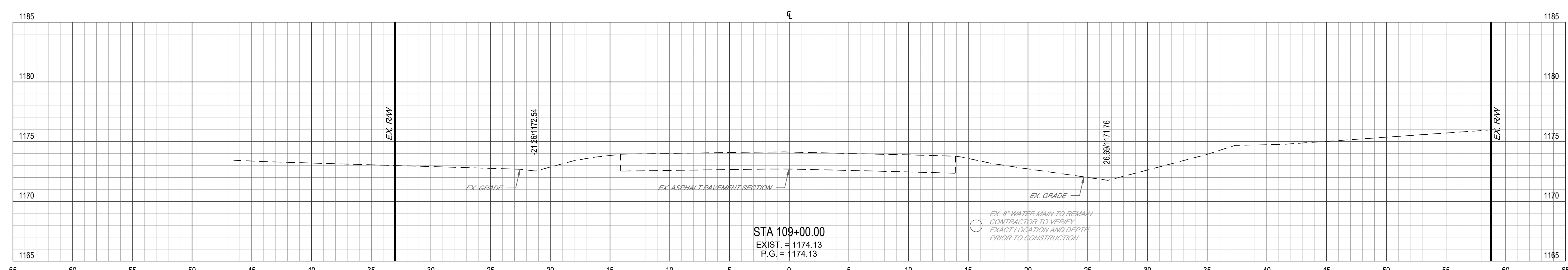
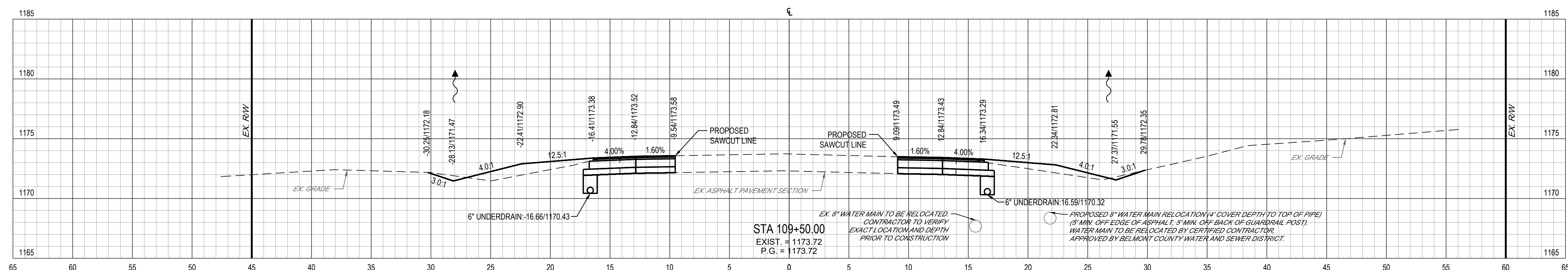
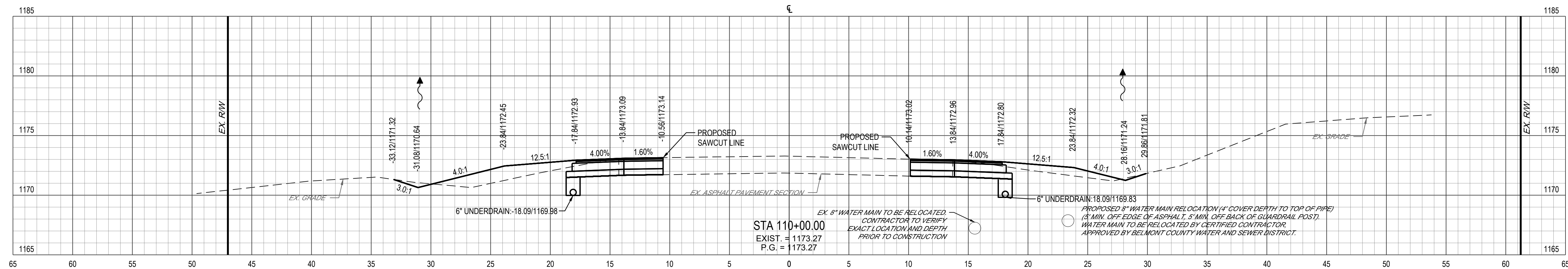
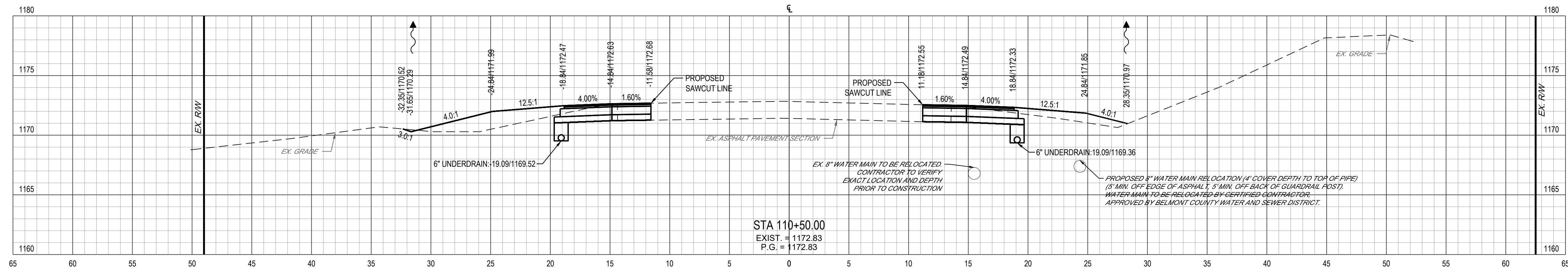


PLAN & PROFILE - RECO DRIVE
S.R. 149

BEL-149-23.44

R16
R31

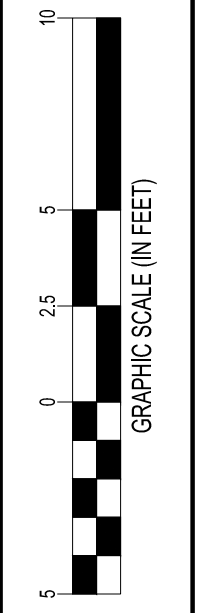
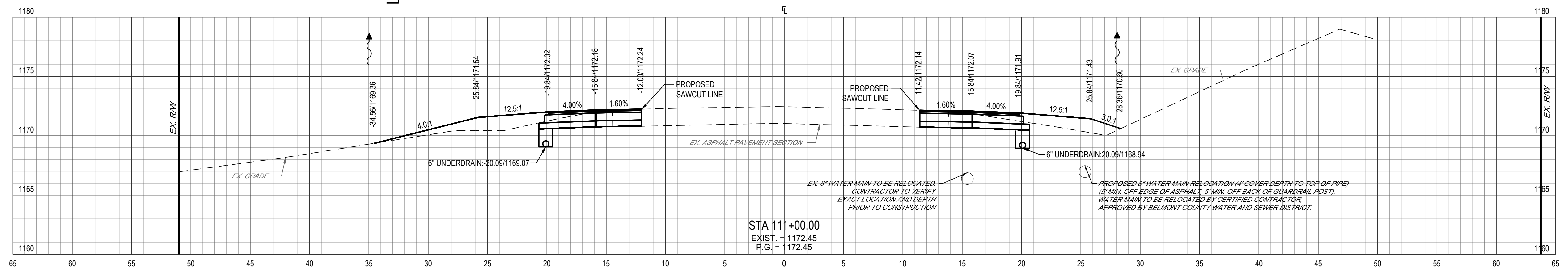
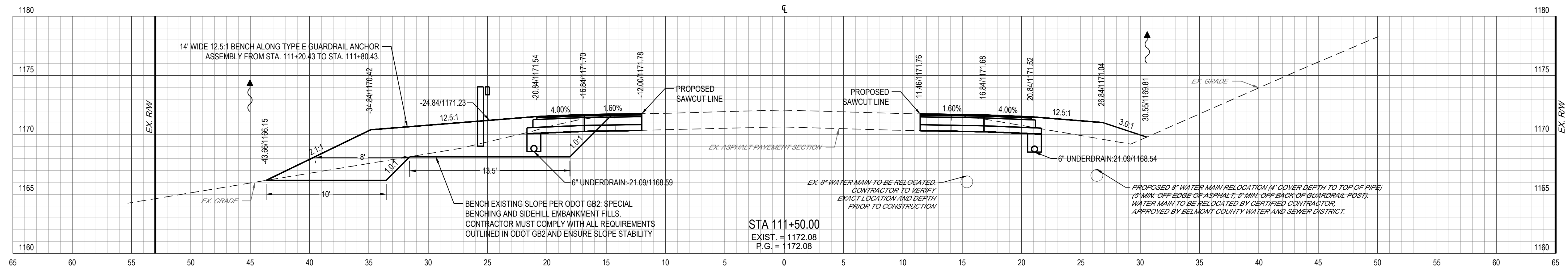
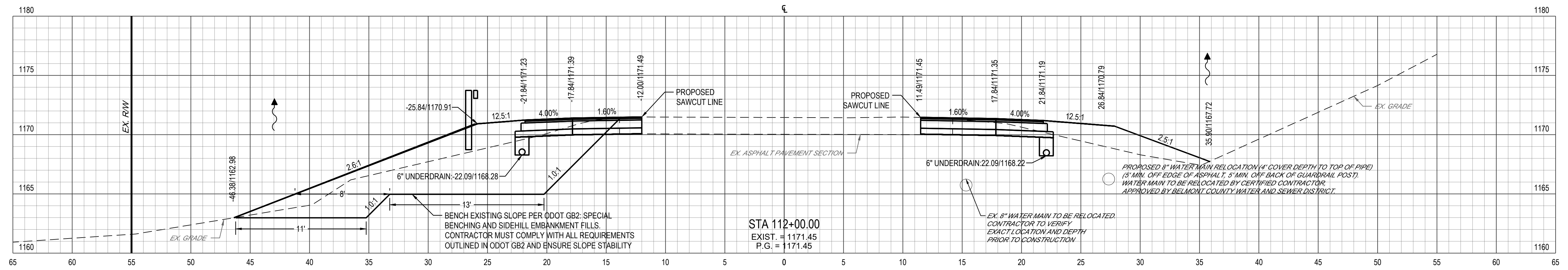
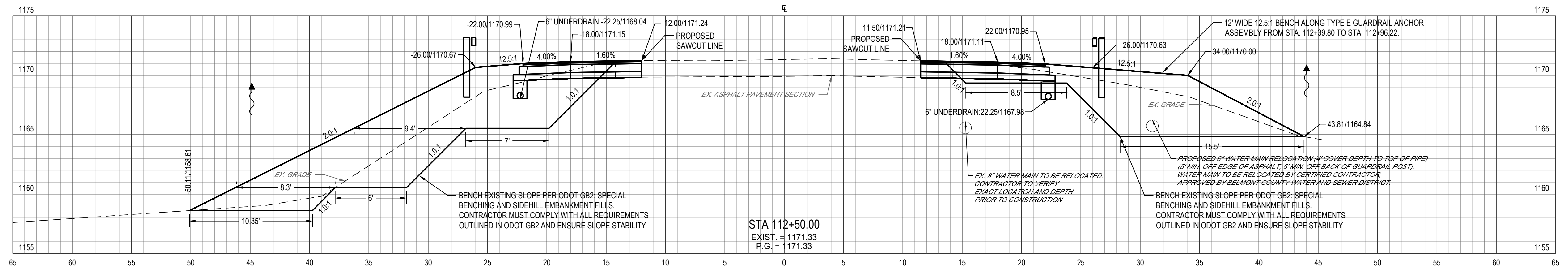
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CROSS SECTIONS - STA. 109+00 TO STA. 110+50
 S.R. 149

BEL-149-23.44

R17
R31



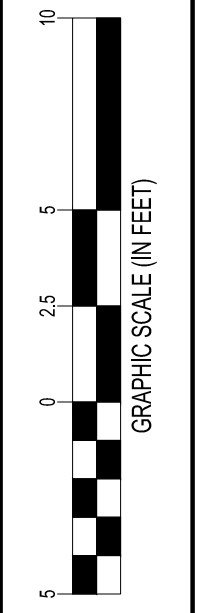
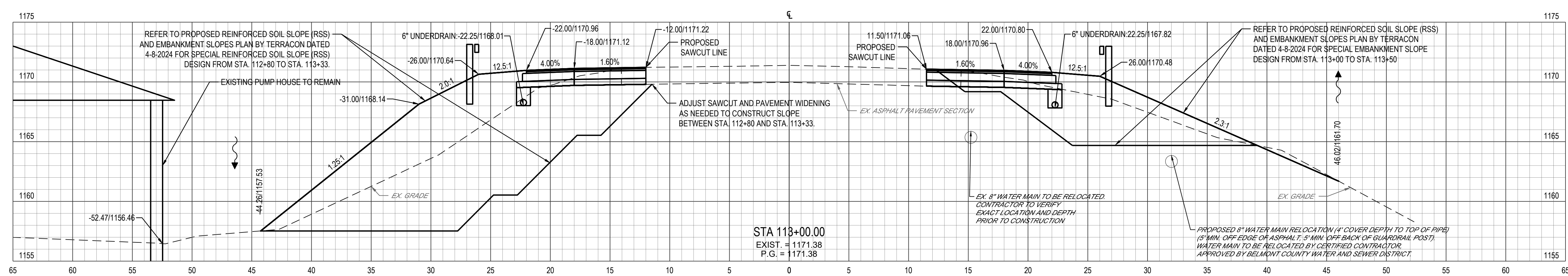
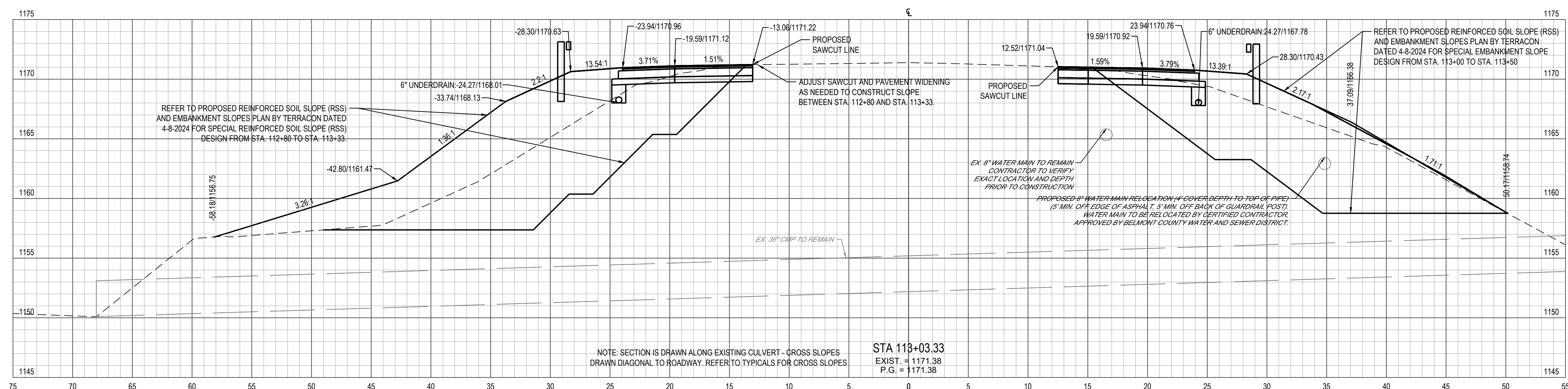
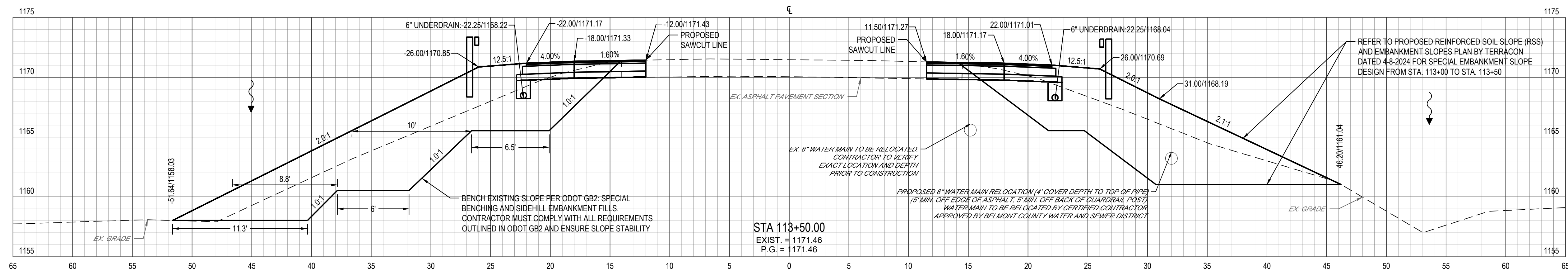
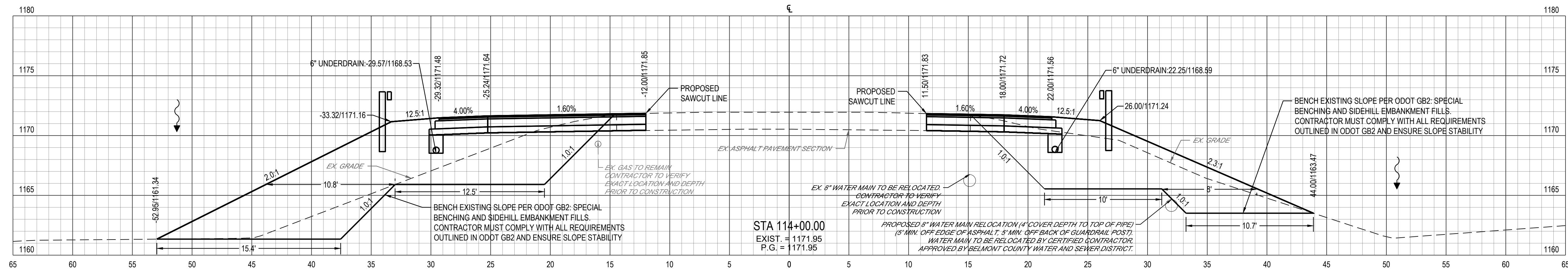
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CROSS SECTIONS - STA. 111+00 TO STA. 112+50
 S.R. 149

BEL-149-23.44

R18
 R31

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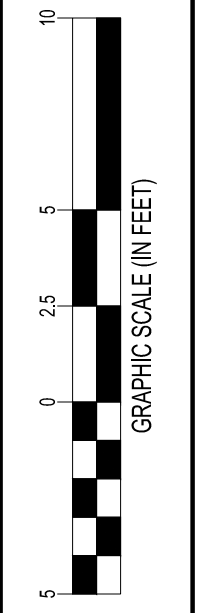
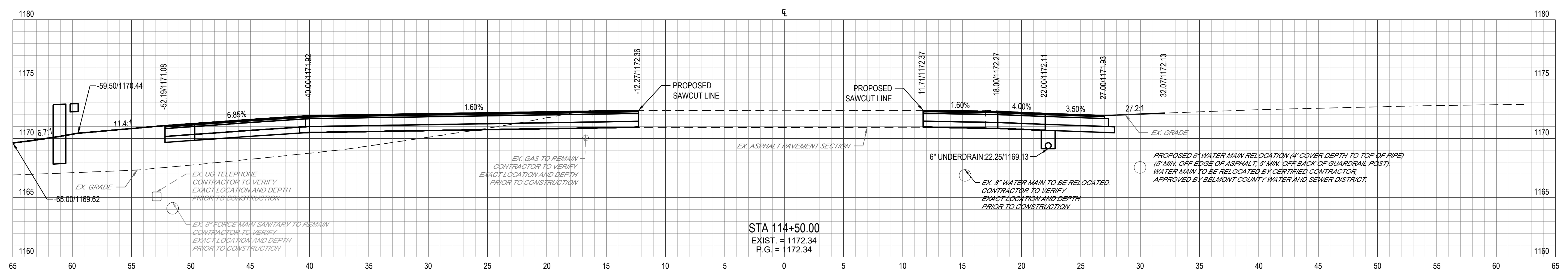
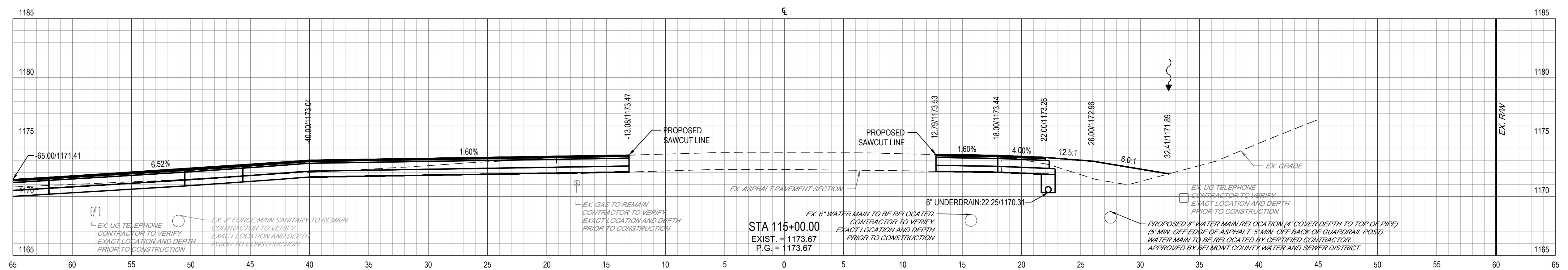
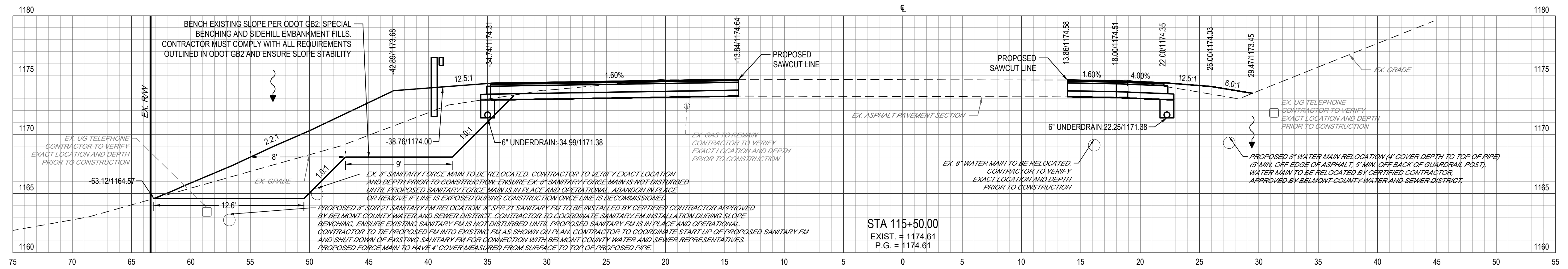
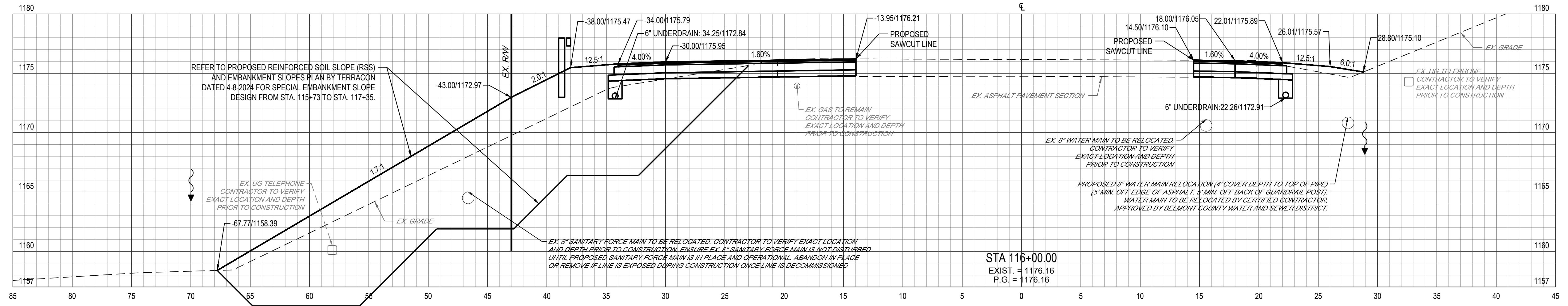
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CROSS SECTIONS - STA. 113+00 TO STA. 114+00
S.R. 149

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R19
R31

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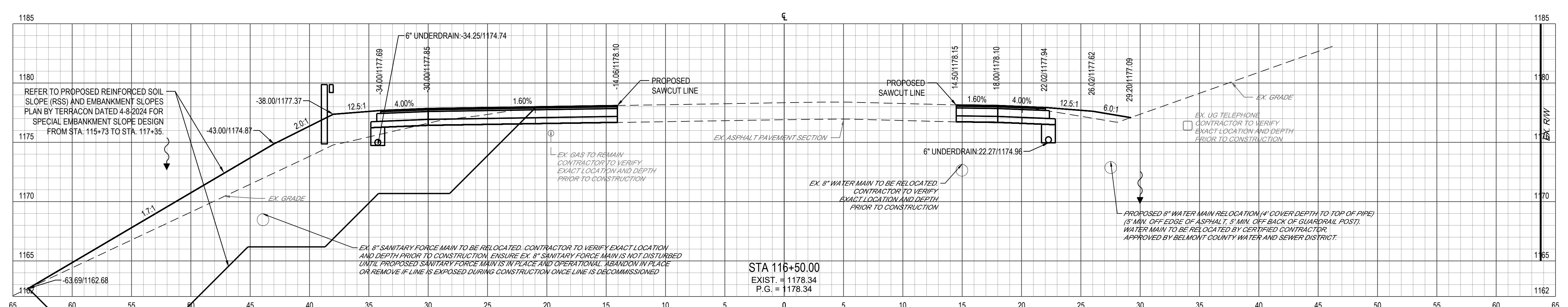
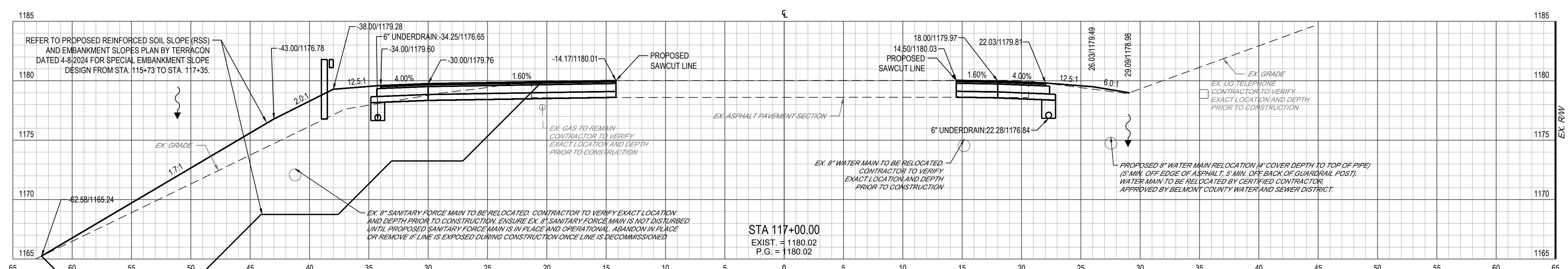
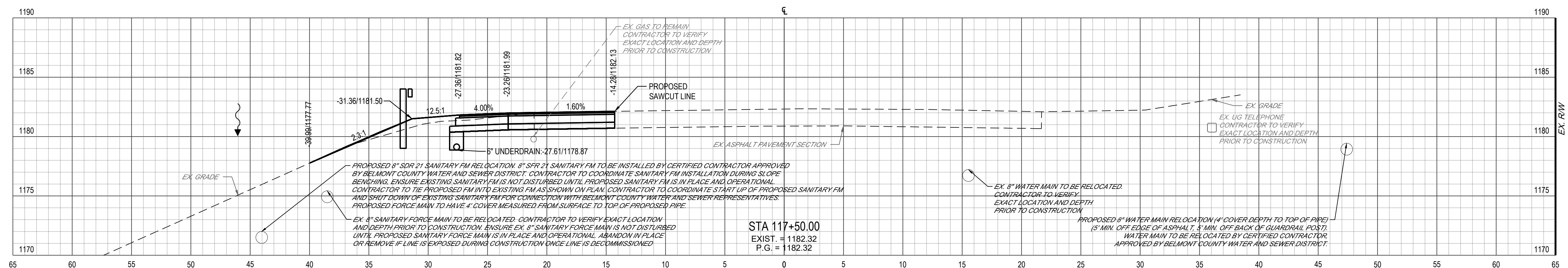
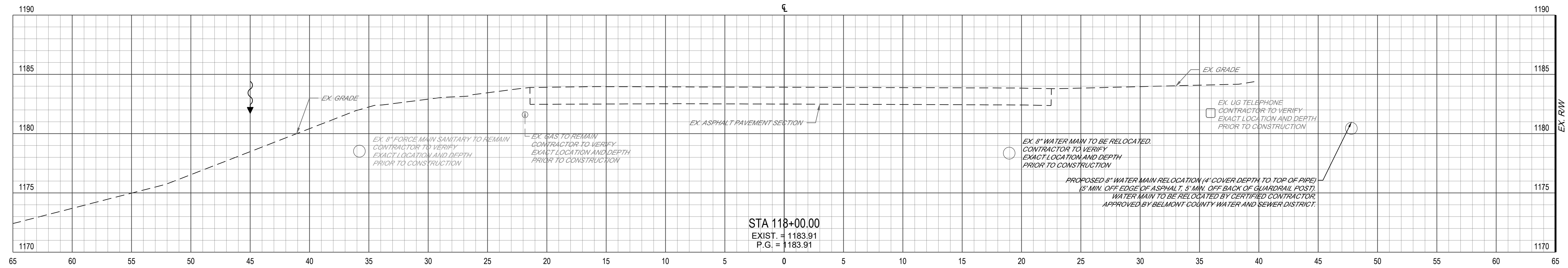
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CROSS SECTIONS - STA. 114+50 TO STA. 116+00
S.R. 149

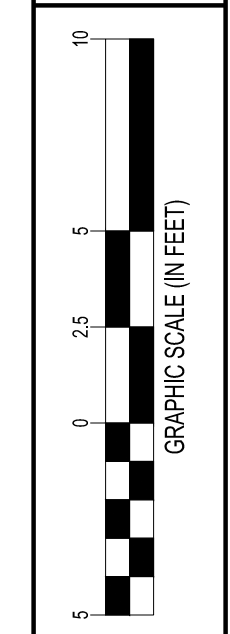
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R20
R31

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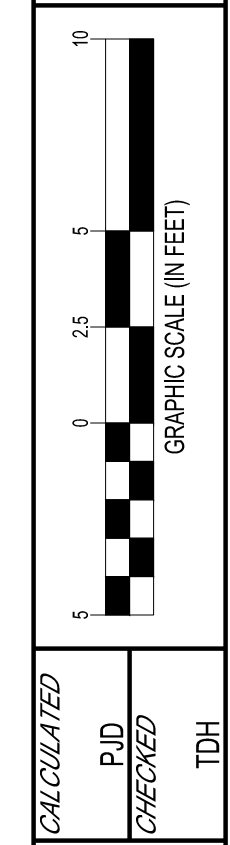
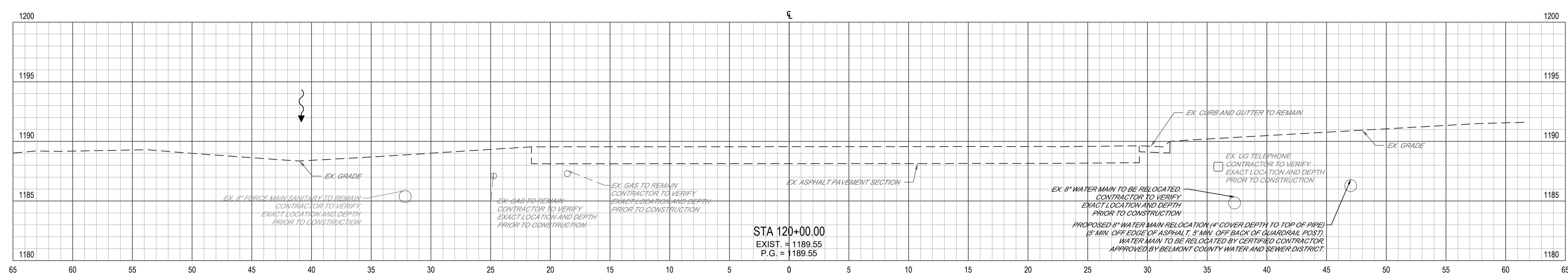
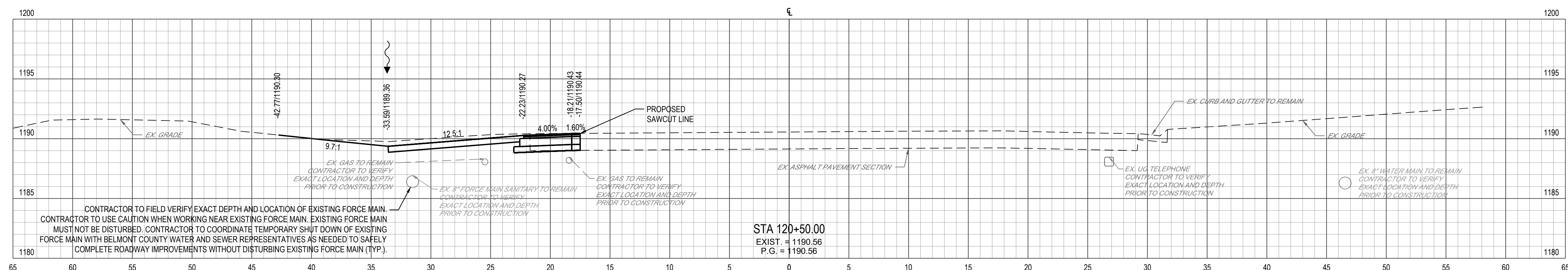
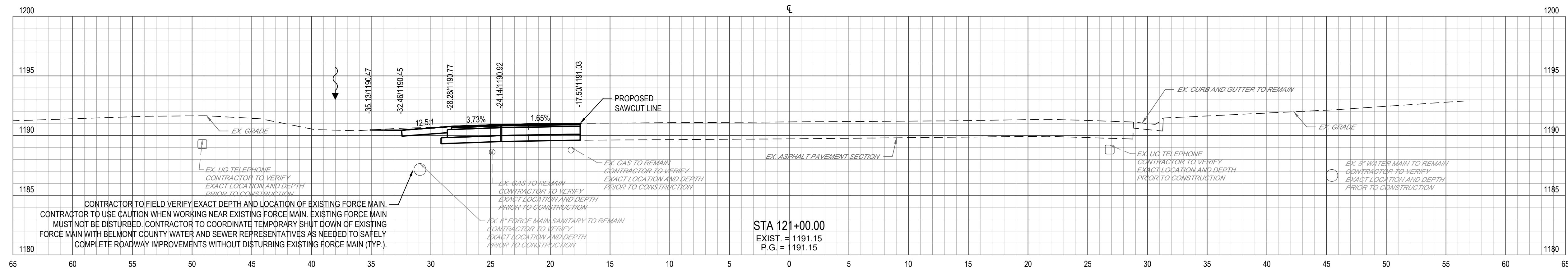
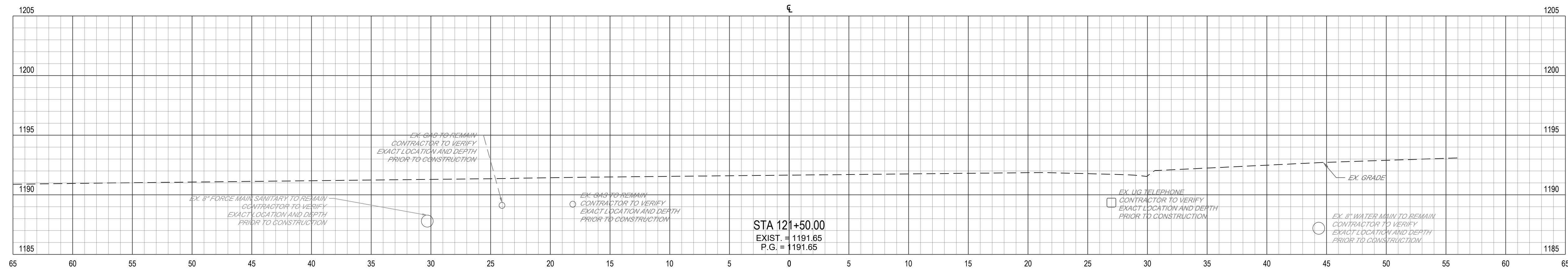


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CROSS SECTIONS - STA. 116+50 TO STA. 118+00
S.R. 149

BEL-149-23.44

R21
R31

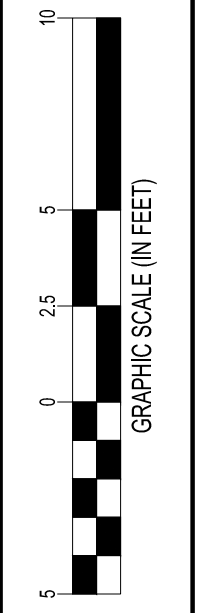
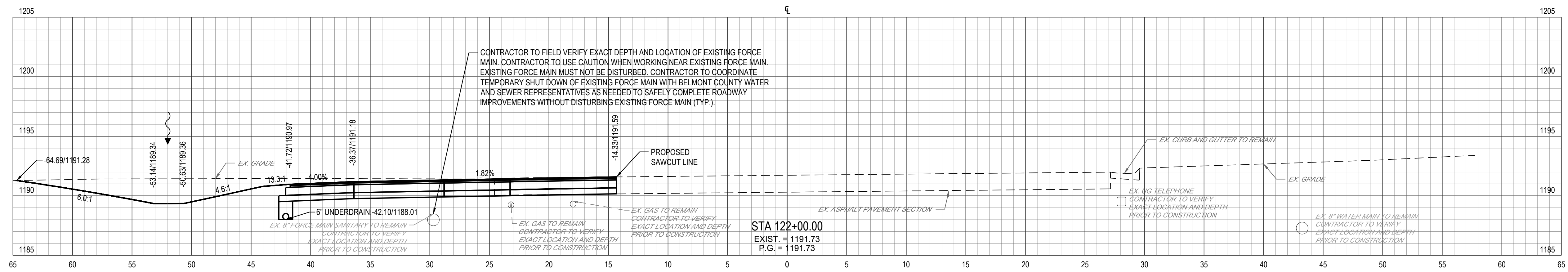
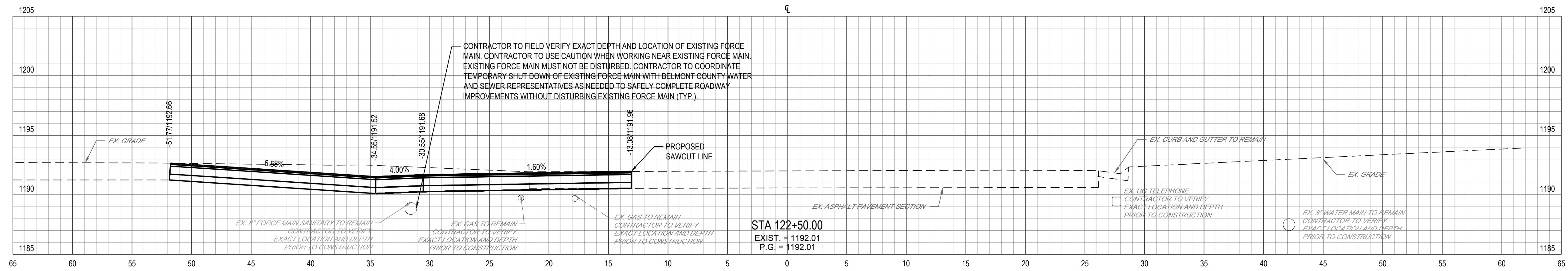
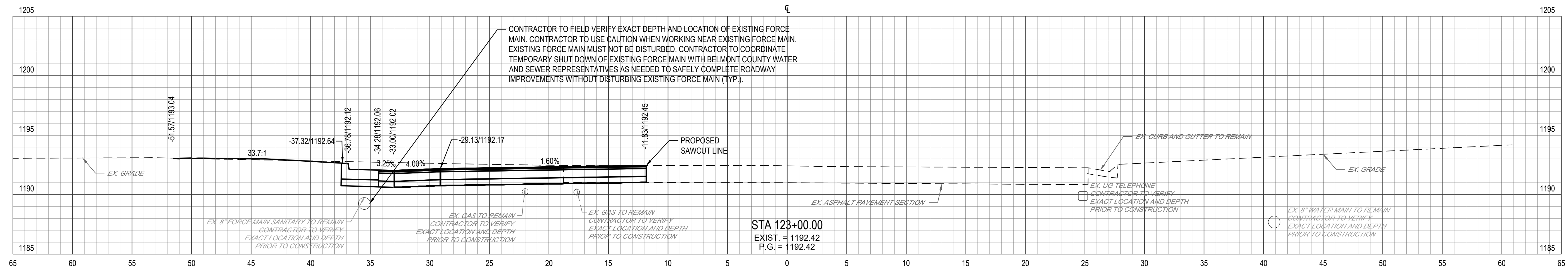
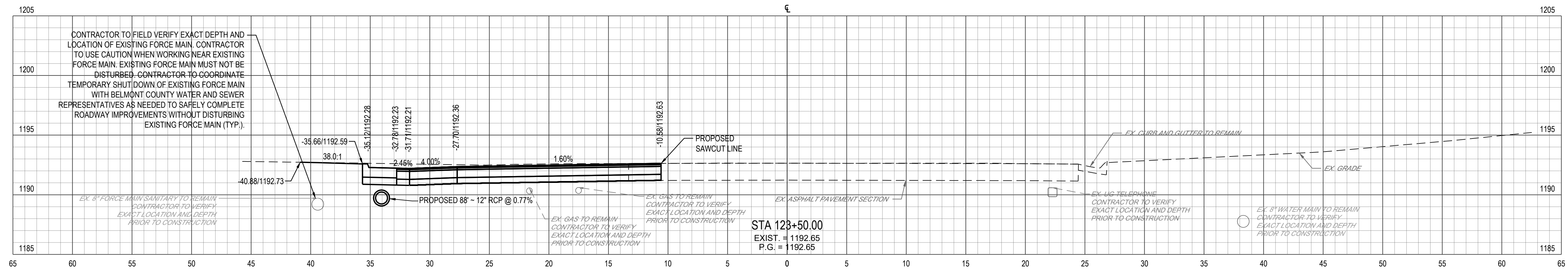


CROSS SECTIONS - STA. 120+00 TO STA. 121+50
 S.R. 149

BEL-149-23.44

R22
R31

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CHECKED		

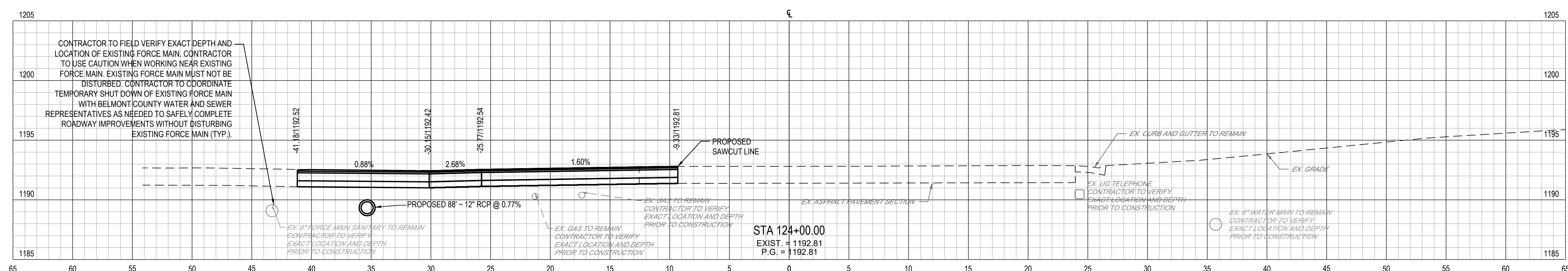
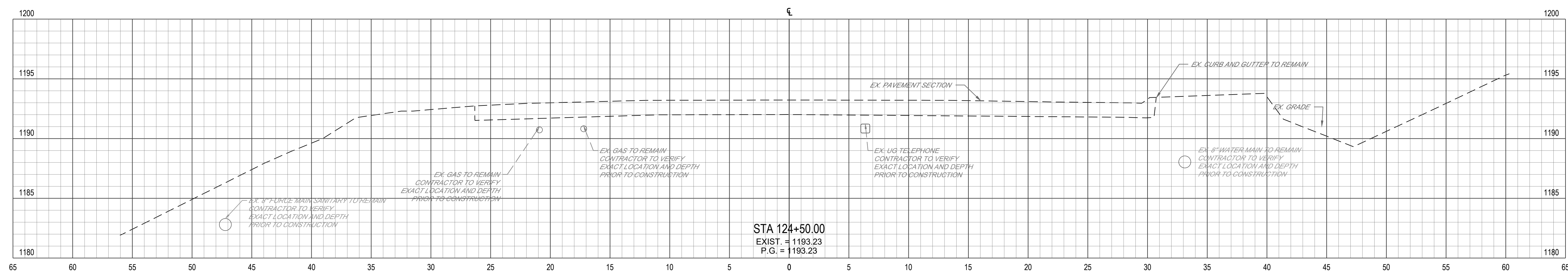
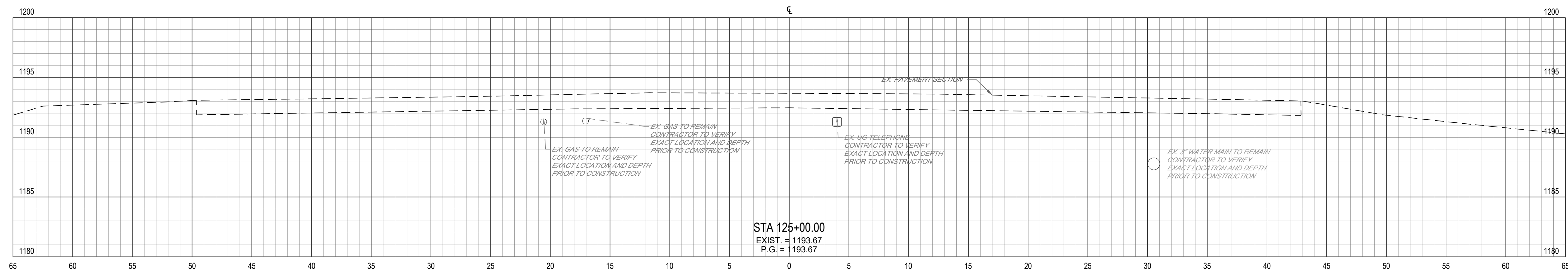
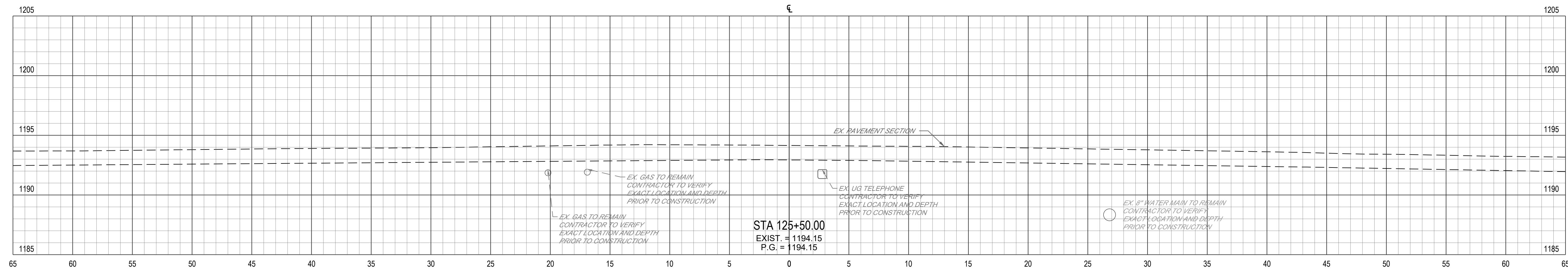
CROSS SECTIONS - STA. 122+00 TO STA. 123+50
S.R. 149

BEL-149-23.44

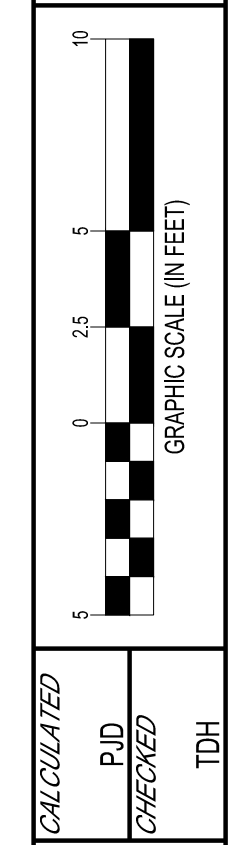
R23
R31

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CONTRACTOR TO FIELD VERIFY EXACT DEPTH AND LOCATION OF EXISTING FORCE MAIN. CONTRACTOR TO USE CAUTION WHEN WORKING NEAR EXISTING FORCE MAIN. EXISTING FORCE MAIN MUST NOT BE DISTURBED. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER REPRESENTATIVES AS NEEDED TO SAFELY COMPLETE ROADWAY IMPROVEMENTS WITHOUT DISTURBING EXISTING FORCE MAIN (TYP.).



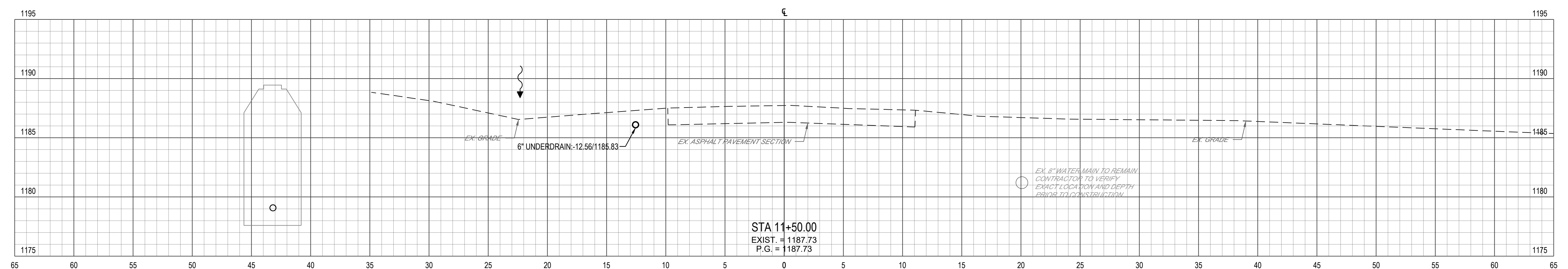
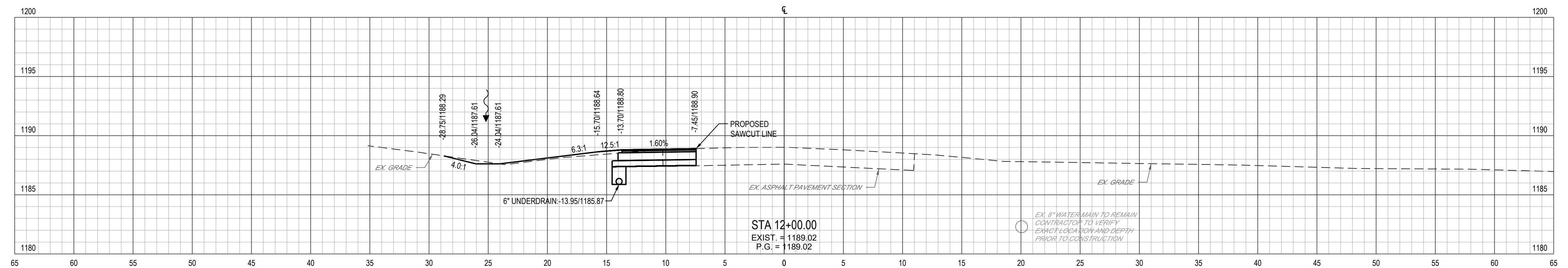
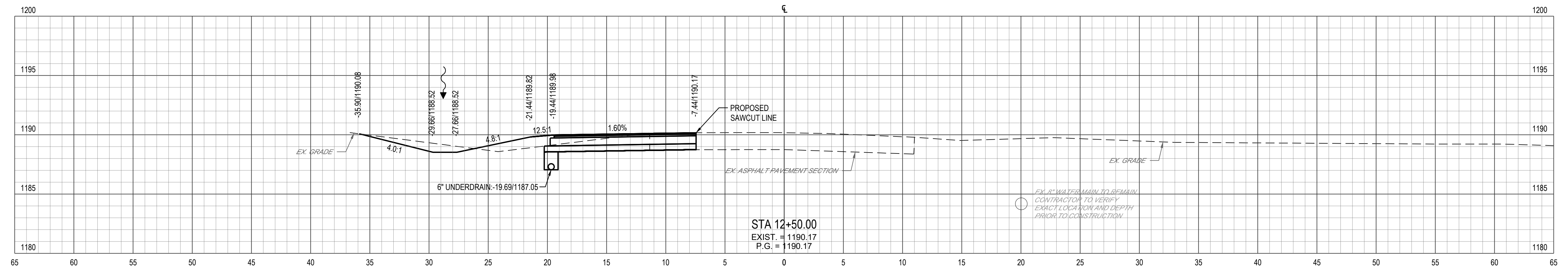
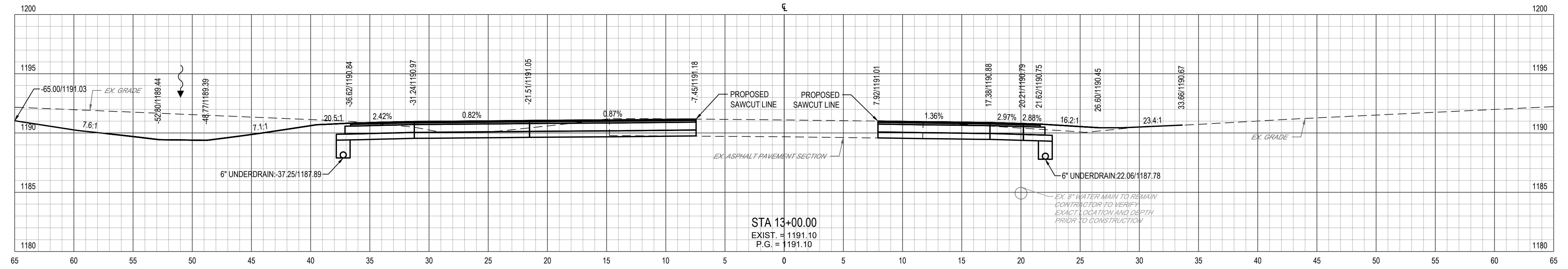
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CROSS SECTIONS - STA. 124+00 TO STA. 125+50
S.R. 149

BEL-149-23.44

R24
R31

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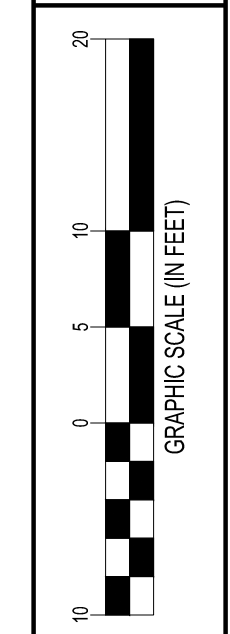
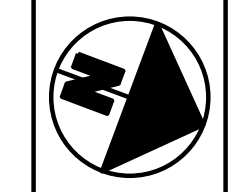
GRAPHIC SCALE (IN FEET)

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CROSS SECTIONS - STA. 11+50 TO STA. 13+00
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BEL-149-23.44

R25
 R31

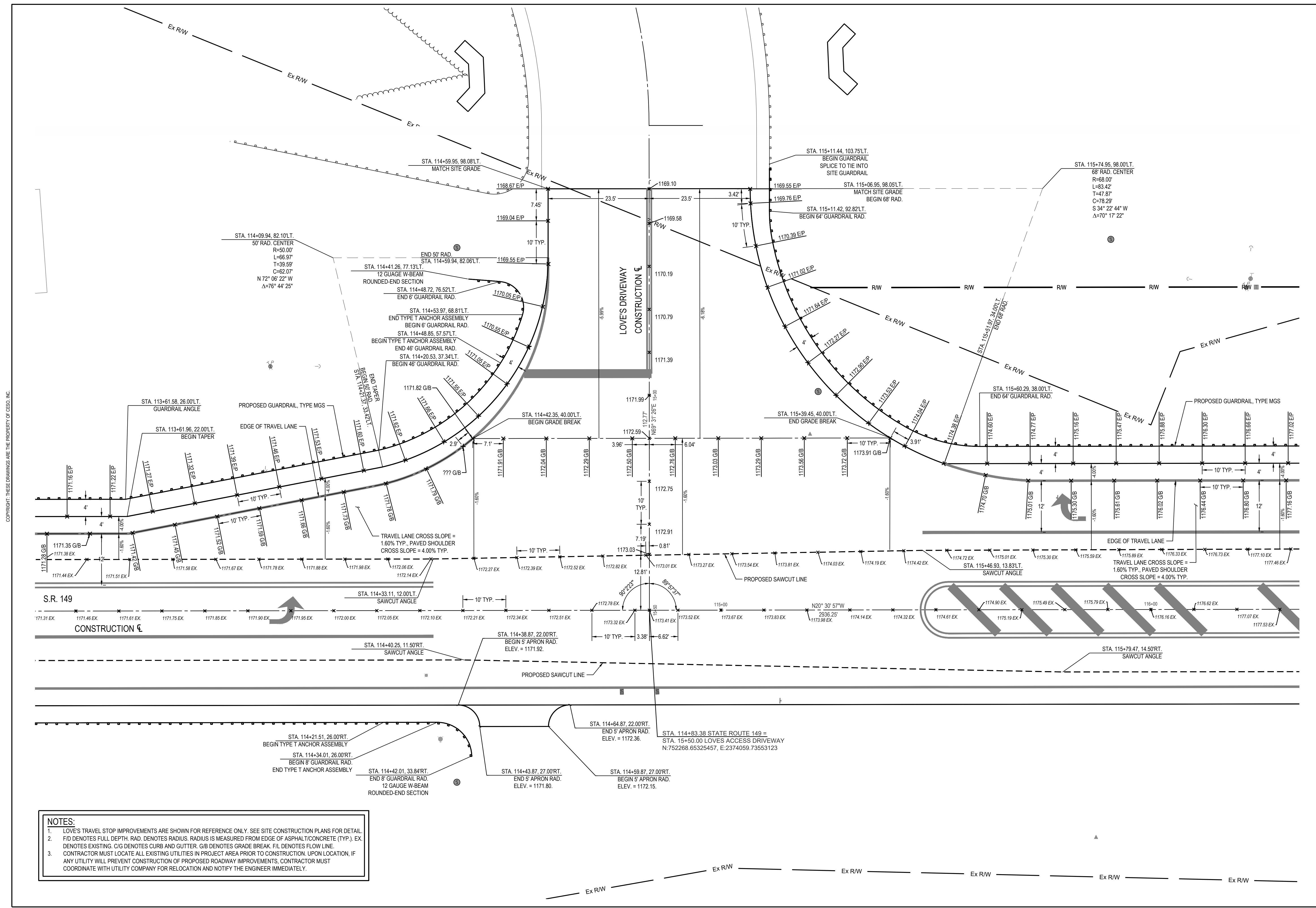


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INTERSECTION DETAIL
S.R. 149

BEL-149-23.44

R26
R31

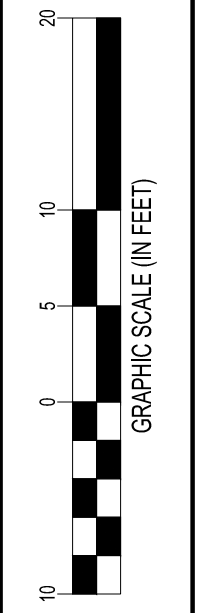
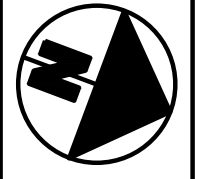


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- NOTES:**
1. LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
 2. F/D DENOTES FULL DEPTH, RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.). EX. DENOTES EXISTING. C/G DENOTES CURB AND GUTTER. G/B DENOTES GRADE BREAK. F/L DENOTES FLOW LINE.
 3. CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.

NOTES:

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- F/D DENOTES FULL DEPTH. RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.). EX. DENOTES EXISTING. C/G DENOTES CURB AND GUTTER. G/B DENOTES GRADE BREAK. F/L DENOTES FLOW LINE. CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.



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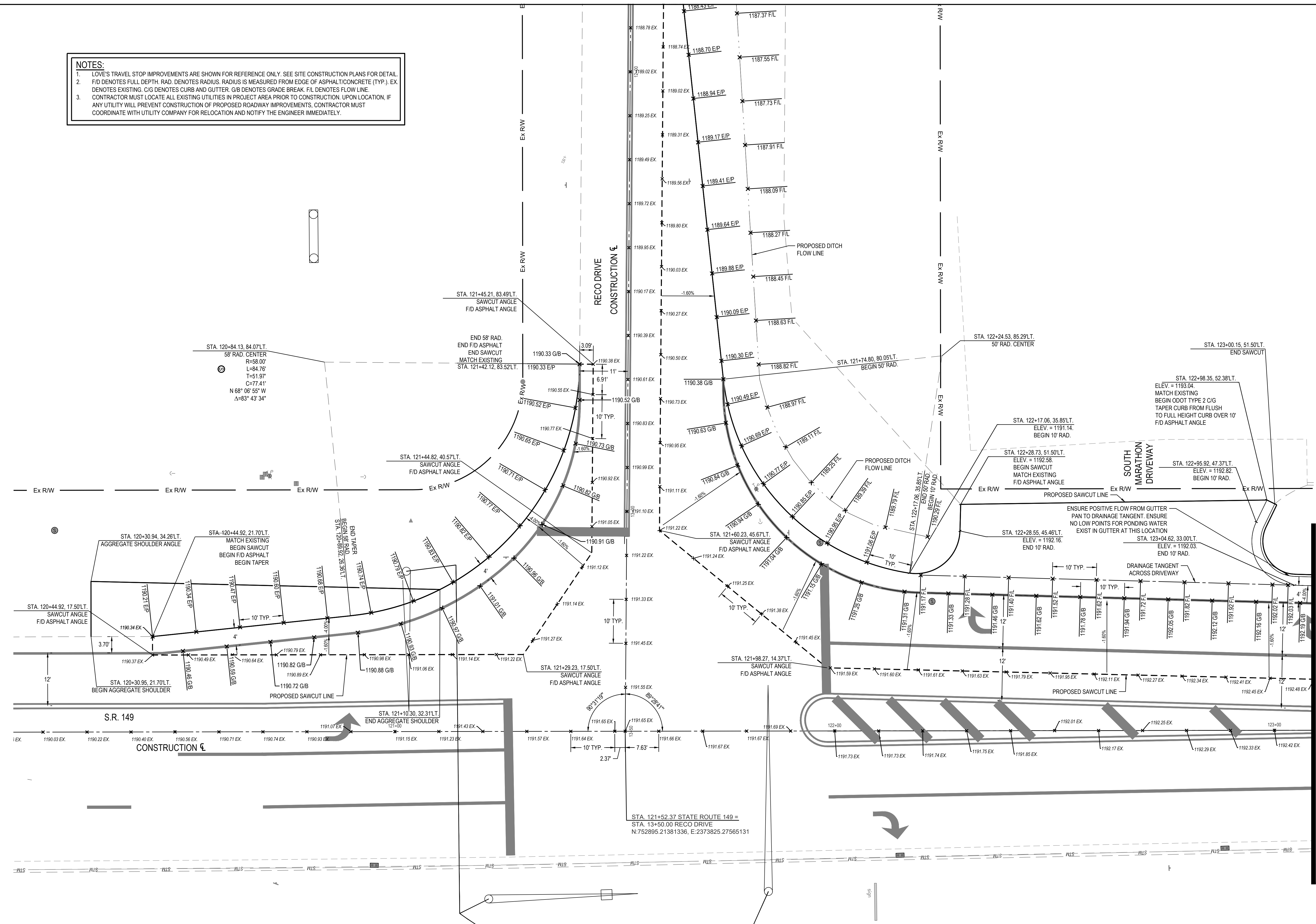
INTERSECTION DETAIL
S.R. 149

MATCHLINE STA 123+10 - SEE SHEET R28

BEL-149-23.44

R27
R31

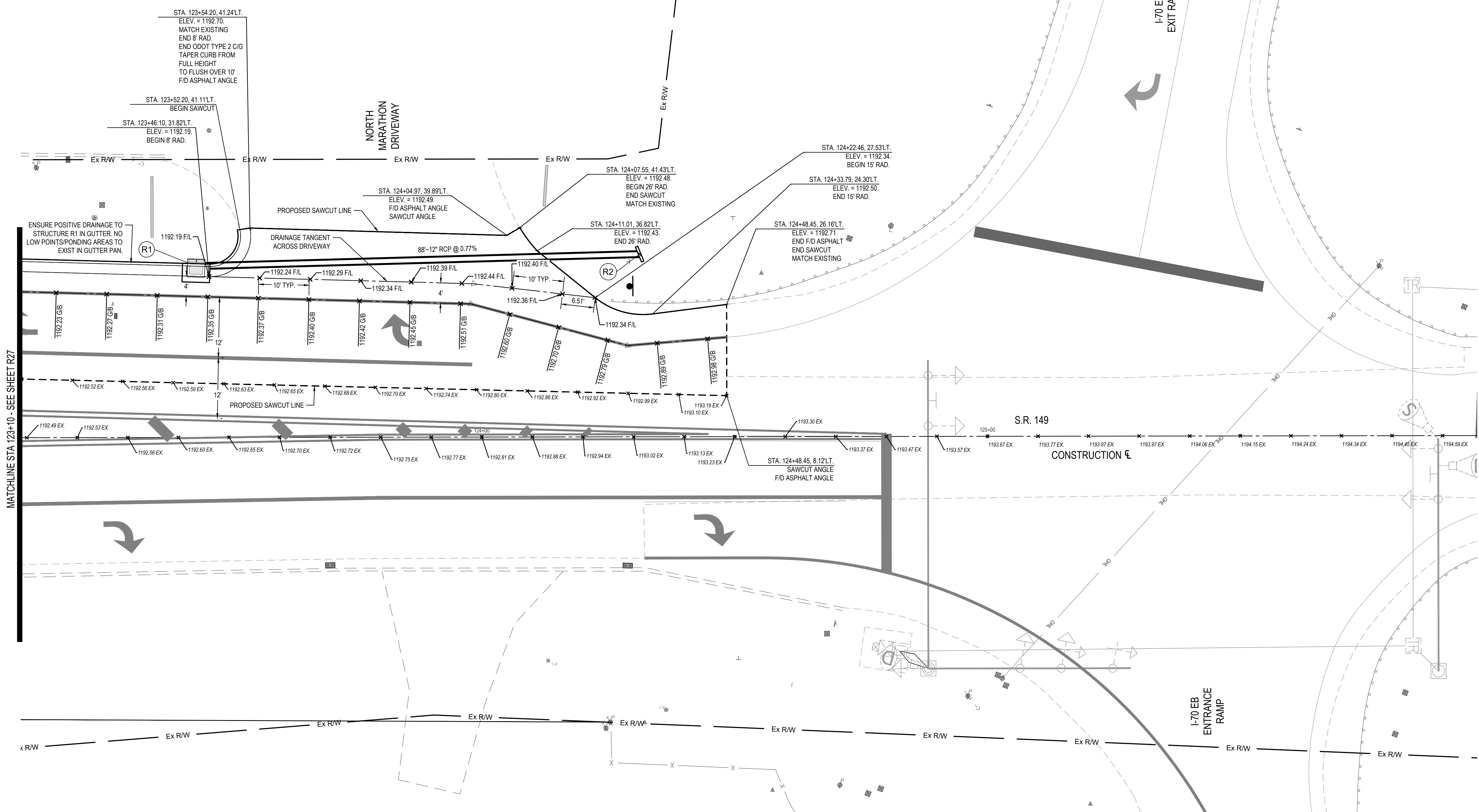
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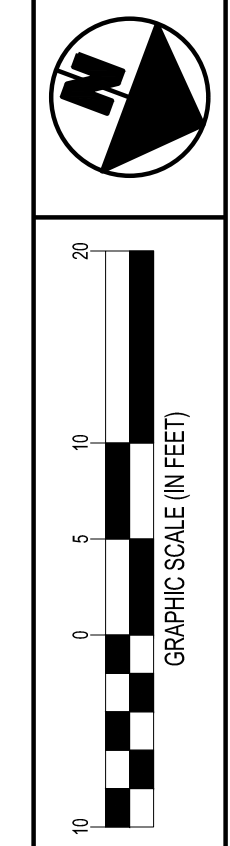
- LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
- F/D DENOTES FULL DEPTH, RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.), EX. DENOTES EXISTING, C/G DENOTES CURB AND GUTTER, G/B DENOTES GRADE BREAK, F/L DENOTES FLOW LINE.
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STORM SEWER STRUCTURE SCHEDULE				
NO.	STRUCTURE	GRATE	INVERT	STATION/OFFSET
R1	CATCH BASIN NO.3A PER ODOT SCD CB-3A	1192.01	1189.28 (12") N	STA. 123+43.57, 33.89'LT.
R2	HALF-HEIGHT HEADWALL PER ODOT SCD HW-2.2		1188.60 (12") S	STA. 124+31.25, 36.15'LT.



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MATCHLINE STA 123+10 - SEE SHEET R27



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INTERSECTION DETAIL
S.R. 149

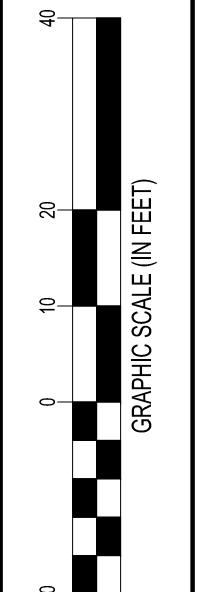
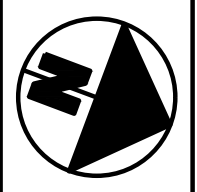
BEL-149-23.44

R28
R31

PAVEMENT MARKING LEGEND

NO.	ITEM	MARKING
1	644	CENTER LINE, 4", DOUBLE YELLOW, SOLID-SOLID
2	644	EDGE LINE, 6", WHITE
3	644	CHANNELIZING LINE, 8", WHITE
4	644	TRANSVERSE/DIAGONAL LINE, 24", YELLOW
5	644	STOP LINE, 24", WHITE
6	644	CENTERLINE, 4", DOUBLE YELLOW, DASHED-SOLID
7	644	LANE ARROW
8	644	LANE LINE, 6", WHITE, 10' LINE, 30' SKIP

CONTRACTOR TO PROVIDE PAVEMENT MARKINGS IN ACCORDANCE WITH THE LATEST VERSION OF THE ODOT STANDARD SPECIFICATIONS AND OHIO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES

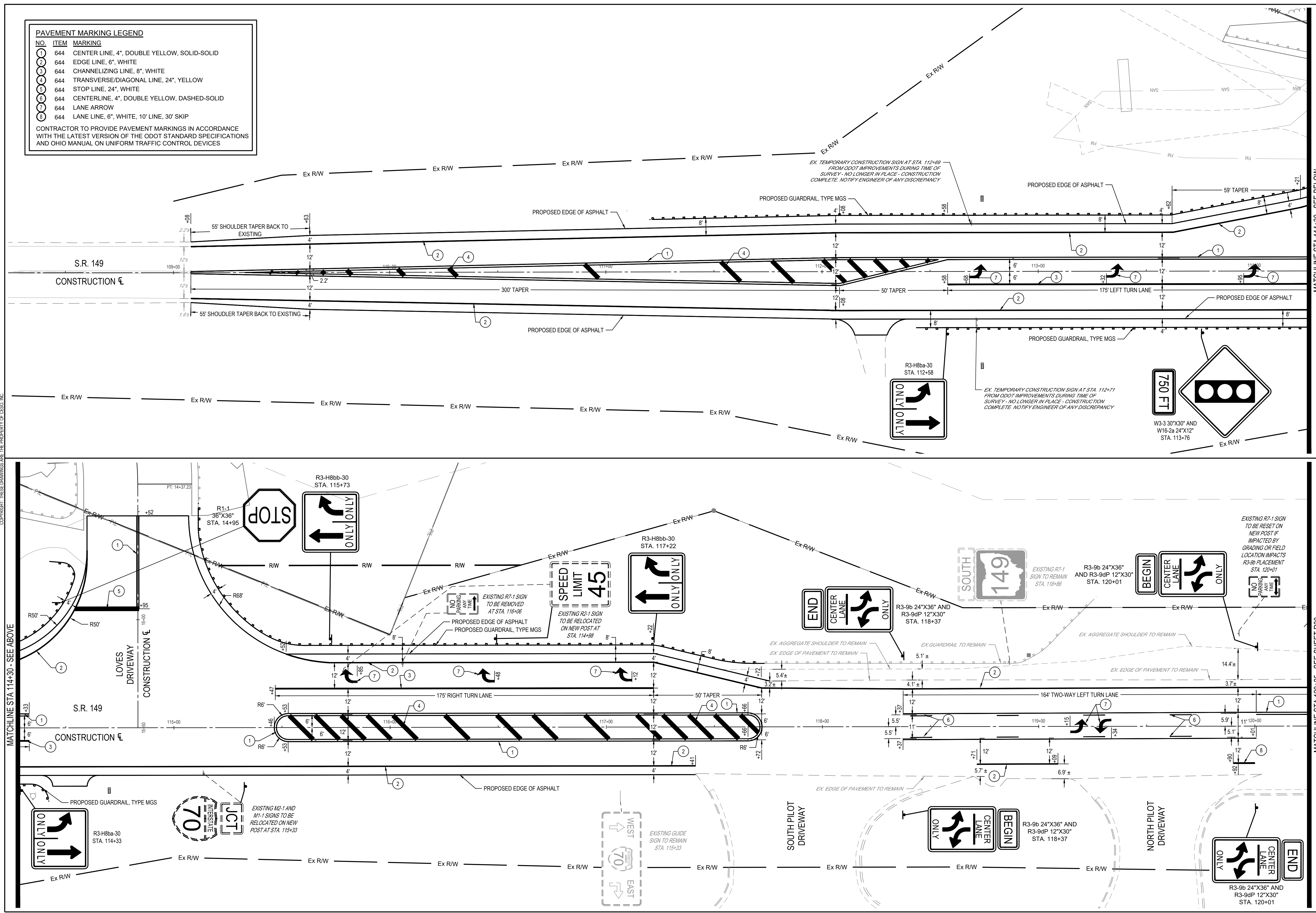


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PAVEMENT MARKING & SIGNAGE - STA. 108+50 TO STA. 120+25
S.R. 149

BEL-149-23.44

R29
R31



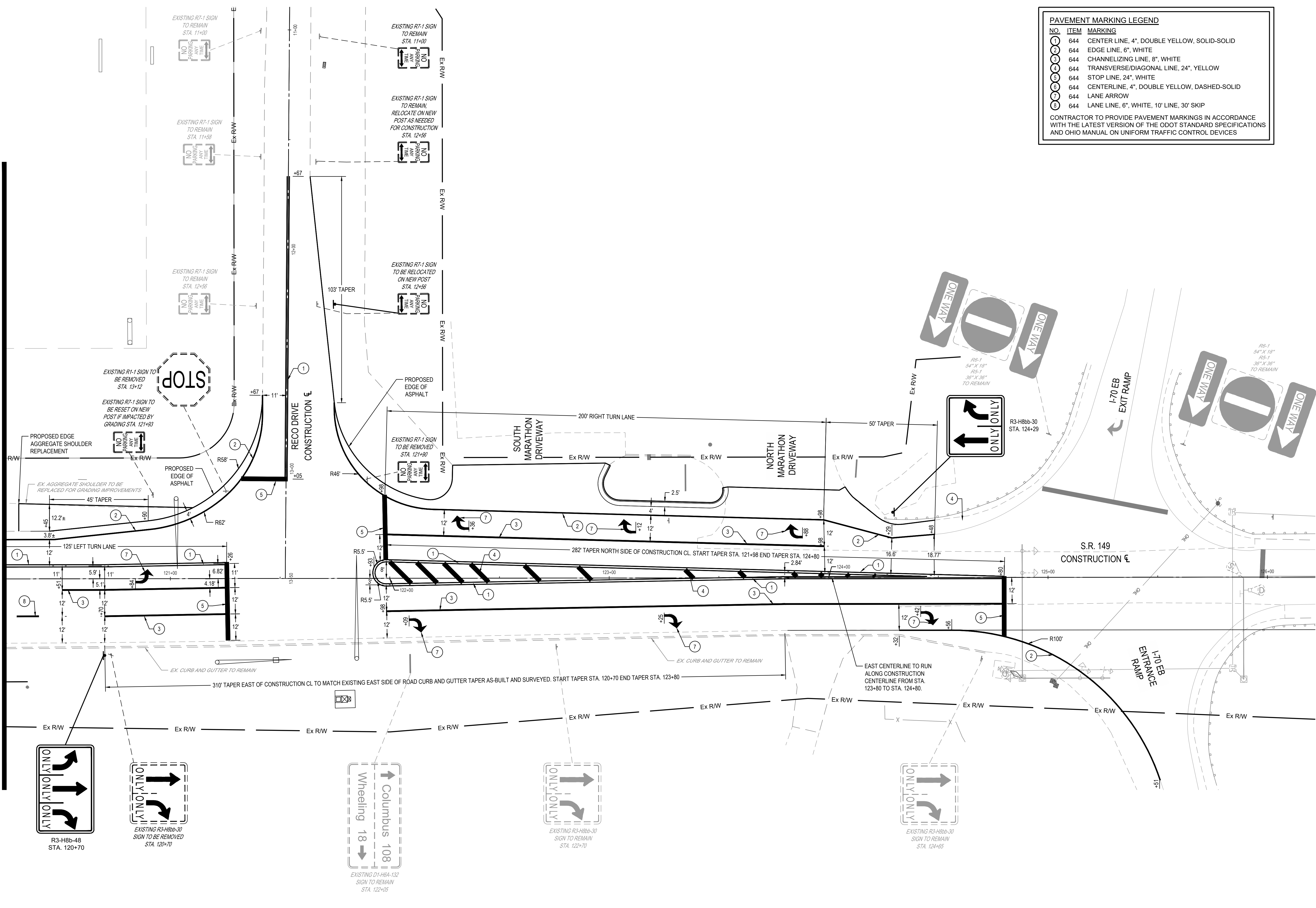
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MATCHLINE STA 114+30 - SEE ABOVE

MATCHLINE STA 120+25 - SEE SHEET R30

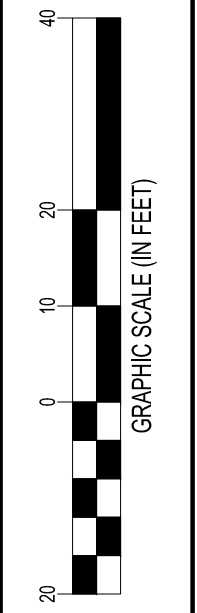
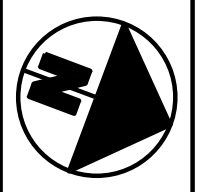
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MATCHLINE STA 120+25 - SEE SHEET R29



PAVEMENT MARKING LEGEND		
1	644	CENTER LINE, 4", DOUBLE YELLOW, SOLID-SOLID
2	644	EDGE LINE, 6", WHITE
3	644	CHANNELIZING LINE, 8", WHITE
4	644	TRANSVERSE/DIAGONAL LINE, 24", YELLOW
5	644	STOP LINE, 24", WHITE
6	644	CENTERLINE, 4", DOUBLE YELLOW, DASHED-SOLID
7	644	LANE ARROW
8	644	LANE LINE, 6", WHITE, 10' LINE, 30' SKIP

CONTRACTOR TO PROVIDE PAVEMENT MARKINGS IN ACCORDANCE WITH THE LATEST VERSION OF THE ODOT STANDARD SPECIFICATIONS AND OHIO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES



CALCULATED
P.I.D.
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PAVEMENT MARKING & SIGNAGE - STA. 120+25 TO 126+00
S.R. 149

BEL-149-23.44



GENERAL CONSTRUCTION REQUIREMENTS	
1. DETAIL PLANS OF PLANNED WATER LINE CONSTRUCTION WILL BE SUBMITTED TO THE BELMONT COUNTY WATER AND SEWER DISTRICT FOR APPROVAL BY THE ENGINEER ASSOCIATE, PRIOR TO CONSTRUCTION. A PRECONSTRUCTION MEETING WILL BE REQUIRED BEFORE START OF CONST.	
2. SIGNED WATER APPLICATIONS & APPLICABLE FEES MUST BE RECEIVED PRIOR TO CONSTRUCTION.	
3. ALL WATERLINES SHALL BE C900 (DR-14) IN HIGH PRESSURE AREAS DUCTILE IRON CLASS 50 OR 52 WILL BE REQUIRED BY THE DISTRICT. (ALL DUCTILE IRON PIPE TO BE POLYETHYLENE WRAPPED AS PER AWWA C105/A21.5) PIPE SIZES TO BE 3/4"CTS, 1"CTS, 2" P.P., 4", 6", 8", 10", 12" C-900 DR 14-18 (3/4" OR 1" TO BE TYPE-K SOFT COPPER IF LINE PRESSURE IS GREATER THAN 200 PSI)	
4. RESIDENTIAL PVC SEWER LINE WILL BE A MINIMUM OF 4 INCH SDR-35.	
5. SEWER FORCE MAIN WILL BE A MINIMUM OF 1 1/2", OR 2" POLYETHYLENE CLASS 200, OR 2", 4", 6", 8" SDR-21 CL-200 PVC PIPE, AND GREEN IN COLOR. GRAVITY SEWER LINES SHALL BE AIR AND DEFLECTION TESTED IN ACCORDANCE WITH TABLE 1 PAGE 29 OF THIS BOOKLET.	
6. ALL PIPE THAT IS INSTALLED SHALL BE BEDDED WITH A MINIMUM OF 6" ABOVE AND BELOW THE PIPE WITH GOOD TOP SOIL FREE OF STONE AND OTHER FOREIGN OBJECTS, OR CRUSH AND RUN STONE.	
7. WATERLINE SHALL BE INSTALLED WITH 4 FOOT MINIMUM COVER UNLESS OTHERWISE APPROVED.	
8. A MINIMUM VERTICAL CLEARANCE OF 18 INCHES SHALL BE PROVIDED BETWEEN THE PROPOSED WATER MAIN AND OTHER UTILITIES OR STRUCTURES, UNLESS OTHERWISE NOTED.	
9. A MINIMUM HORIZONTAL CLEARANCE OF 4 FEET SHALL BE PROVIDED BETWEEN THE PROPOSED WATER LINE AND OTHER UTILITIES OR STRUCTURES UNLESS OTHERWISE APPROVED.	
10. THE WATERLINE SHALL BE PLACED WITH A MINIMUM OF 10 FOOT HORIZONTAL SEPARATION FROM ANY SANITARY SEWERS OR WHEN THE WATER LINE CROSSES A SANITARY SEWER, THE WATER LINE SHALL BE A MINIMUM OF 18 INCHES ABOVE THE SEWER.	
11. SEWER LINES MUST CROSS UNDER ALL WATERLINES WITH A MINIMUM OF 18" OF CLEARANCE. IF NOT POSSIBLE THE BCSD WILL PERMIT CROSSING OVER WITH STEEL CASING OR PLASTIC CASING AND CONCRETE. MINIMUM CASING LENGTH IS 10FT. AS PER NOTE ON SHT.- #16	
12. ALL FITTINGS SHALL BE MECHANICAL JOINT (M.J), FUSION EPOXY COATED, DUCTILE IRON, RESTRAINED BY THE USE OF RESTRAINERS AND/OR MEGALUGS AND THRUST BLOCKED WITH CONCRETE AS REQUIRED. SEE SHT# 25 FOR THRUST BLOCKING DETAIL.	
13. A 16FT. MIN. LENGTH OF WATERLINE SHALL BE PROVIDED ON EACH SIDE OF ANY MAINLINE FITTING, OR THE JOINT SHALL BE RESTRAINED WITH BELL RESTRAINT FITTINGS.	
14. ALL SERVICE CONNECTIONS MADE TO EXISTING LINES WILL BE MADE BY THE DISTRICT. THE CONNECTION SHALL BE MADE WET USING A TAPPING TEE AND VALVE. IF A LINE TO BE CONNECTED IS THE SAME AS THE EXISTING LINE, 2" CONNECTED TO 2" OR 6" CONNECTED TO 6". THE CONNECTION WILL BE MADE DRY. EXCEPTIONS MAY BE MADE DUE TO UNFORESEEN FIELD CIRCUMSTANCES.	
CONTINUES ON NEXT PAGE	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
12/14/17	2

GENERAL CONSTRUCTION REQUIREMENTS	
15. MAGNETIC DETECTABLE TRACER TAPE SHALL BE INSTALLED AT 18 INCHES DEEP WITH ALL PVC PIPE.	
16. TRACE WIRE TO BE INSTALLED ON ALL WATER & SEWER LINES.	
A.) OPEN-TRENCH INSTALLATION: DIRECT BURIAL #12 AWG SOLID (0.0808" DIA.), STEEL CORE SOFT DRAWN TRACER WIRE, 250# AVERAGE TENSILE BREAK LOAD, 30 MIL HIGH MOLECULAR-HIGH DENSITY POLYETHYLENE JACKET COMPLYING WITH ASTM-D-1248.30 VOLT RATING, 45 MIL JACKET WHEN DIRECTIONAL BORED.	
B.) CONNECTORS SPICES ALONG THE CONTINUOUS RUN OF TRACE WIRE FOR REPAIR OF A WIRE BREAK OR REPLACEMENT OF FAILED SEGMENT OF WIRE SHALL USE 3M BRAND DBR DIRECT BURY SPLICE KIT OR APPROVED EQUAL.	
C.) INSTALLATION TRACE WIRE ACCESS POINT SHALL BE ACCESSIBLE AT ALL NEW WATER VALVE BOXES, WATER METER BOXES, BLOWOFFS, ARVs, FIRE HYDRANTS AND MANHOLES.	
D.) TESTING REQUIREMENTS CONTRACTOR SHALL PERFORM A CONTINUITY TEST ON ALL TRACE WIRE IN THE PRESENCE OF BCSD. IF THE TRACE WIRE IS FOUND TO BE NOT CONTINUOUS AFTER TESTING, CONTRACTOR SHALL REPAIR OR REPLACE THE FAILED SEGMENT OF WIRE.	
17. THE CONTRACTOR WILL SCHEDULE WITH THE DISTRICT APPROXIMATE TIMES FOR THE INSPECTION OF LINES, FITTINGS & ETC., PRIOR TO BACKFILLING. THE COUNTY SHALL BE GIVEN A MINIMUM 48 HOURS OF NOTICE WHEN AN INSPECTION IS REQUIRED.	
18. EXISTING MAINLINE VALVES WILL ONLY BE OPERATED BY DISTRICT PERSONNEL.	
19. A MINIMUM OF 24 HOURS NOTICE FOR ANY INSTALLATIONS BY CONTRACTORS THAT MAY DISRUPT WATER SERVICE TO EXISTING WATER CUSTOMERS. THE NOTICE SHALL BE MADE THROUGH THE DISTRICTS OFFICE.	
20. ALL SERVICE CONNECTIONS INSTALLED WILL BE STAKED AT THE TIME OF CONSTRUCTION WITH IRON "TEE BAR STAKES" PER DISTRICT REQUIREMENTS.	
21. CHANGES IN GRADE AND ALIGNMENT TO BE OBTAINED BY DEFLECTIONS AT PIPE JOINTS SHALL BE NO GREATER THAN RECOMMENDED BY THE MANUFACTURER OF THE WATER LINE.	
22. PRESSURE TESTING OF WATER LINES AND SEWER FORCE MAINS SHALL BE IN ACCORDANCE WITH AWWA C600-87. TESTING AT 1-1/2 TIMES EXISTING PRESSURE AND/OR WORKING PRESSURE OR MINIMUM OF 100psi/MAX. OF 200 psi. PRESSURE TESTING MUST BE DONE WITHIN 2 WORKING WEEKS ONCE LINE IS IN PLACE.	
23. THE CONTRACTOR WILL CHLORINATE THE INSTALLED WATER LINE AT RECOMMENDED AWWA 651 STANDARDS, FOR A MINIMUM OF 24 HOURS, AT 50 P.P.M.	
24. THE DISTRICT WILL TAKE REPRESENTATIVE SAMPLES OF THE WATER IN THE LINES, AND HAVE AN APPROVED LABORATORY MAKE THE BACTERIOLOGICAL EXAMINATIONS OF THE SAMPLE TO ESTABLISH THE ACCEPTABILITY OF THAT PORTION OF THE SYSTEM PRIOR TO ANY TURN ON OF SERVICE TO ANY FACILITY. THERE WILL BE NO EXCEPTION TO THIS STANDARD.	
CONTINUES ON NEXT PAGE	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
10/31/17	3

GENERAL CONSTRUCTION REQUIREMENTS	
25. ALL MAIN LINE VALVES & FLUSH-OUT ASSEMBLIES SHALL BE MARKED BY A WHITE FIBERGLASS MARKER AS PER THE WATER DISTRICTS REQUIREMENTS.	
26. LOCATION OF THE PROPOSED SERVICE TAPS SHALL BE FIELD LOCATED BY THE OWNER PRIOR TO CONSTRUCTION OR AT TIME OF CONSTRUCTION.	
27. ALL MATERIAL AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BELMONT COUNTY SPECIFICATIONS AS OUTLINED IN STANDARD DETAILS MANUAL.	
28. ALL PARTS, LABOR, EQUIPMENT, AND MISC. EXPENSES BY THE BELMONT COUNTY SANITARY SEWER DISTRICT WILL BE INVOICED TO THE DEVELOPER.	
29. CONTRACTOR IS RESPONSIBLE FOR ALL WORK PERFORMED FOR THE FIRST YEAR OF CUSTOMER SERVICE.	
30. ALL PRESSURE TESTING STARTS BEFORE NOON.	
31. AS-BUILT DRAWINGS (MAPS) MUST BE PROVIDED TO BCSD WITHIN 3 MONTHS OF THE BACTERIA TEST, WHICH MARKS THE WATER LINE COMPLETION DATE. FOR 6" AND LARGER LINES/ AND WITHIN ONE MONTH FOR 2" & 4" LINES.	
32. AS-BUILT FOR 6" & LARGER LINES ARE REQUIRED TO HAVE GPS POINT WHEN SUBMITTED FOR APPROVAL THEY ARE TO POINT OUT THE FOLLOWING: HYDRANTS, FITTINGS, CURB BOXES, METER WELLS, MAN HOLES, CLEAN OUTS, VALVES, AND WATER OR SEWER LINES IN 100FT. INCREMENTS.	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
10/31/17	3A

NOT TO SCALE	
NOTE: TEE BAR STAKES TO MARK CURB BOX ON ALL VACANT AND OR UNDEVELOPED PROPERTY.	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
9/25/17	STANDARD DETAILS INSIDE SERVICE SETTING DWG. NO. 2

NOT TO SCALE	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
10/31/17	STANDARD DETAILS VALVE BOX SETTING DWG. NO. 5

NOT TO SCALE	
<ul style="list-style-type: none"> FIRE HYDRANT--DRESSER MH STYLE 929---TRAFFIC MODEL NATIONAL STANDARD NOZZLE THREADING---SHOE CONNECTION 6" M.J. PENTAGON OPERATING NUT---DIRECTION OF OPENING COUNTER CLOCKWISE MIN. 3" BURY---5-1/4" VALVE OPENING---BRONZE TO BRONZE HYDRANT TO BE PAINTED YELLOW EXTENSIONS MAY BE NECESSARY WHENEVER HYDRANTS ARE PLACED AT TOE OF SLOPE. (TO BE DETERMINED BY DISTRICT PERSONNEL) 	
BELMONT COUNTY WATER AND SEWER DISTRICT BELMONT COUNTY, OHIO	
10/31/17	STANDARD DETAILS FIRE HYDRANT ASSEMBLY ARRANGEMENT AT END OF WATERLINE EXTENSION DWG. NO. 7

CONSTRUCTION DETAILS
S.R. 149

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ITEM 632 - POWER SERVICE, AS PER PLAN

ELECTRIC POWER SHALL BE OBTAINED FROM THE AEP AT THE LOCATION INDICATED ON THE PLANS. POWER SUPPLIED SHALL BE 120 VOLTS.

POWER SERVICE SHALL BE AS PER CMS ITEM 632 AND SCD TC- 83.10 WITH THE FOLLOWING EXCEPTIONS:

1. THE CONTRACTOR SHALL MEET WITH A REPRESENTATIVE FROM THE POWER SUPPLY AGENCY TO CONFIRM HOW THE PROPOSED POWER SERVICE IS TO BE WIRED, HOOKED UP, AND ITS LOCATION

2. ALL POWER SERVICES SHALL BE METERED. THE METER SHALL HAVE A LEVER OPERATED BYPASS.

DISCONNECT SWITCH ENCLOSURES FURNISHED IN ACCORDANCE WITH CMS ITEM 632, POWER SERVICE, AS PER PLAN, SHALL INCLUDE A PADLOCK EQUAL TO MASTER NO. 4BKA OR WILSON BOHANNON 660, WITH LOCK BODY OF BRONZE OR BRASS AND KEYING SHALL BE TO THE STATE MASTER. THE CONTRACTOR SHALL CONTACT THE METER SECTION OF THE POWER COMPANY FOR INFORMATION REGARDING THE METER BASE INSTALLATION PRIOR TO ORDERING POLES. CONTRACTOR WILL BE RESPONSIBLE FOR REQUESTING AND SCHEDULING ANY INSPECTIONS THE POWER COMPANY MAY REQUIRE FOR THE POWER SERVICE HOOK UP. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONTACT THE POWER COMPANY FOR THE ELECTRICAL SERVICE CONNECTION. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR SPLICE POWER CABLE INTO THE POWER COMPANY'S CIRCUITS. THE VOLTAGE SUPPLIED SHALL BE NOMINALLY 120 VOLTS. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ANY NECESSARY PERMITS AND THE PAYING OF ALL FEES. THE CONTRACTOR SHALL PAY ALL POWER CHARGES UNTIL THE SIGNAL IS ACCEPTED BY MAINTAINING AGENCY.

SIGNAL ACTIVATION

PRIOR TO ACTIVATION THE NEW TRAFFIC SIGNAL TO STOP-AND- GO MODE AND/OR REMOVING THE EXISTING TRAFFIC SIGNAL FROM SERVICE, ALL ITEMS IN THE PROPOSED SIGNAL PLAN SHALL BE FULLY COMPLETED, (I.E., VEHICLE DETECTION, PEDESTRIAN SIGNAL HEADS, ETC.). IF THERE ARE CONSTRUCTABILITY ISSUES (I.E., ROADWAY WIDENING, ETC.) THAT PREVENT THE SIGNAL FROM BEING COMPLETED PRIOR TO ACTIVATION, IT SHALL BE BROUGHT TO THE ATTENTION OF THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER. THE DISTRICT TRAFFIC ENGINEER WILL THEN REVIEW, APPROVE OR REJECT PROPOSALS TO ACTIVATE THE TRAFFIC SIGNAL PRIOR TO COMPLETION.

THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER AT LEAST 10 WORKING DAYS PRIOR TO SCHEDULING THE FINAL INSPECTION OF THE SIGNAL INSTALLATION. FINAL INSPECTION IS NOT CONSIDERED COMPLETE UNTIL DESIGNATED DISTRICT TRAFFIC PERSONNEL INSPECT THE TRAFFIC SIGNAL AND ISSUE WRITTEN APPROVAL. IF ISSUES ARE FOUND DURING THE FINAL INSPECTION THAT EFFECT THE SAFETY OF THE TRAVELING PUBLIC AND/OR THE EFFICIENCY OF THE INTERSECTION, THE SIGNAL SHALL NOT BE ACTIVATED OF THE PROPOSED DATE. ANY PUNCH LIST ITEMS THAT ARE FOUND SHALL BE CORRECTED AND REINSPECTED BY DISTRICT TRAFFIC PERSONNEL PRIOR TO FINAL ACCEPTANCE. ODOT FORCES SHALL ONLY ASSUME DAY TO DAY MAINTENANCE OF THE TRAFFIC SIGNAL AFTER FINAL WRITTEN ACCEPTANCE HAS BEEN ISSUED.

ITEM 632 - SIGNAL SUPPORT FOUNDATION

PRIOR TO ORDERING THE SIGNAL SUPPORTS, THE CONTRACTOR SHALL CONTACT OUPS TO HAVE ALL THE UTILITIES LOCATED IN THE FIELD. THEN, THE CONTRACTOR SHALL MEET WITH THE PROJECT ENGINEER TO LOCATE THE PROPOSED SUPPORT LOCATIONS TO INSURE THERE ARE NO CONFLICTS WITH UTILITIES. IF THERE ARE ISSUES, THE PROJECT ENGINEER SHALL PROVIDE GUIDANCE AS TO THE RELOCATION OF THE SUPPORT POLES.

DUE TO THE FURTHER POSSIBILITY OF CONFLICT WITH EXISTING OR PROPOSED UNDERGROUND OBSTRUCTIONS (INCLUDING THE POSSIBILITY OF UNRECORDED OBSTRUCTIONS) WHICH COULD AFFECT THE LOCATION OF THE FOUNDATION FOR THIS ITEM, AND CONSEQUENTLY, THE DESIGN OF THE SUPPORT AND/OR ARMS, THE CONTRACTOR SHALL NOT PLACE FINAL ORDERS FOR THE ITEM UNTIL THE FOUNDATIONS HAVE BEEN INSTALLED, AT FINAL GRADE, AND THE CONTRACTOR HAS RECEIVED, FROM ENGINEER, WRITTEN NOTICE TO PROCEED WITH THE ORDERS FOR THE ITEM.

IF ANY FOUNDATION LOCATIONS MUST BE ADJUSTED, THE CONTRACTOR SHALL NOTIFY THE ENGINEER AND MAINTAINING AGENCY, WHO WILL DETERMINE THE REVISED LOCATION AND IF NEEDED, THE SUPPORT DESIGN. THE CONTRACTOR WILL NOT BE RESPONSIBLE FOR DETERMINING THE REVISED DESIGN. THE ENGINEER WILL INFORM THE CONTRACTOR OF ANY CHANGES NECESSARY AND AUTHORIZE THE CONTRACTOR TO ORDER THE SUPPORT.

THE CONTRACTOR SHALL, WHEN DEVELOPING THE PROGRESS SCHEDULE, AND THOSE OF SUBCONTRACTORS, ENSURE THAT THE FOUNDATIONS ARE INSTALLED AT THE EARLIEST TIME AS IS FEASIBLE AND PRACTICAL, AND SHALL INCLUDE SUFFICIENT TIME IN THE PROGRESS SCHEDULE FOR ORDERING, MANUFACTURING, DELIVERY, AND INSTALLATION OF THE SUPPORT ITEMS AFTER THE FOUNDATIONS ARE IN PLACE.

NO PAYMENTS FOR DELIVERED MATERIALS FOR THE FOUNDATION OR SUPPORT ITEMS SHALL BE MADE UNTIL THE FOUNDATIONS ARE IN PLACE, AND IF CHANGES IN THE DESIGN OF THIS ITEM ARE REQUIRED, NO PAYMENT SHALL BE MADE FOR THE ITEMS MANUFACTURED TO THE ORIGINAL DESIGN.

PAYMENT WILL BE AT THE CONTRACT UNIT PRICE AND WILL BE FULL COMPENSATION FOR ALL LABOR, MATERIALS, TOOLS, EQUIPMENT AND OTHER INCIDENTALS NECESSARY FOR EACH SUPPORT FURNISHED, IN PLACE, COMPLETE AND ACCEPTED.

UNDERDRAINS FOR PULL BOXES

REFERENCE TRAFFIC SCD HL-30.11 FOR DETAILS ABOUT DRAINING PULL BOXES. UNDERDRAINS FOR PULL BOXES SHALL BE USED AS DIRECTED BY THE ENGINEER AND SHALL BE PROVIDED WHERE THE LENGTH REQUIRED FOR A SATISFACTORY OUTLET DOES NOT EXCEED 20 FEET. THE FOLLOWING ESTIMATED QUANTITY IS CARRIED TO THE GENERAL SUMMARY FOR THIS PURPOSE:

ITEM 611 - 4" CONDUIT, TYPE E FOR UNDERDRAIN OUTLET 10 FT

WORK INSPECTION

THE CONTRACTOR SHALL PROVIDE THE PROJECT ENGINEER AND THE DISTRICT TRAFFIC ENGINEER WITH 72 HOUR NOTICE OF ANY SIGNAL WORK TO BE PERFORMED AT THE INTERSECTION SITE(S) SO THAT INSPECTION SERVICES CAN BE SUPPLIED.

GUARANTEE

THE CONTRACTOR SHALL GUARANTEE THAT THE TRAFFIC CONTROL SYSTEM INSTALLED AS PART OF THIS CONTRACT SHALL OPERATE SATISFACTORILY FOR A PERIOD OF 90 DAYS FOLLOWING COMPLETION OF THE 10-DAY PERFORMANCE TEST. IN THE EVENT OF UNSATISFACTORY OPERATION THE CONTRACTOR SHALL CORRECT FAULTY INSTALLATIONS, MAKE REPAIRS AND REPLACE DEFECTIVE PARTS WITH NEW PARTS OF EQUAL OR BETTER QUALITY.

EQUIPMENT, MATERIAL AND LABOR COSTS INCURRED IN CORRECTING AN UNSATISFACTORY OPERATION SHALL BE BORNE BY THE CONTRACTOR.

THE GUARANTEE SHALL COVER THE FOLLOWING ITEMS OF THE TRAFFIC CONTROL SYSTEM: CONTROLLER, CABINET, UNINTERRUPTIBLE POWER SUPPLY, VEHICLE DETECTION EQUIPMENT, LED LAMP UNITS, NETWORK AND COMMUNICATION/INTERCONNECT EQUIPMENT.

CUSTOMARY MANUFACTURER'S GUARANTEES FOR THE FOREGOING ITEMS SHALL BE TURNED OVER TO ODOT DISTRICT 11 FOLLOWING ACCEPTANCE OF THE EQUIPMENT.

THE COST OF GUARANTEEING THE TRAFFIC CONTROL SYSTEM WILL BE INCIDENTAL TO AND INCLUDED IN THE CONTRACT UNIT PRICE OF THE VARIOUS ITEMS MAKING UP THE SYSTEM.

ITEM 625 - ARC FLASH CALCULATIONS AND LABEL

THIS ITEM SHALL INCLUDE PROVIDING ARC FLASH HAZARD CALCULATIONS AND LABELS PER SUPPLEMENTAL SPECIFICATION 825. LABELS SHALL BE APPLIED TO THE OUTSIDE OF EACH CONTROLLER CABINET. LABELS SHALL BE PROVIDED BY THE ODOT SIGN SHOP.

1606 WEST BROAD STREET
COLUMBUS, OHIO 43223

LOCATIONS OF ARC FLASH HAZARD LABELS ARE:

SIGNAL CONTROLLER (STA. 120+90.62, 36.58' LT.)

PAYMENT WILL BE MADE AT THE UNIT PRICE BID FOR EACH ITEM 625 "ARC FLASH CALCULATION LABEL" FOR EACH SET OF CALCULATIONS ACCEPTED AND LABEL INSTALLED, INCLUDING ALL LABOR, MATERIALS, AND INCIDENTALS NECESSARY.

ITEM 633 - CABINET, TYPE 332L, AS PER PLAN

THE CABINET SHALL BE FURNISHED AND INSTALLED ACCORDING TO CMS 633 AND 733, AND BE LISTED ON THE TRAFFIC AUTHORIZED PRODUCTS LIST (TAP).

THE CABINET SHALL BE FURNISHED WITH AN EDI MONITOR AS ALLOWED ON THE TAP/APPROVED PRODUCTS LIST. THE CABINET SHALL INCLUDE A CABINET RISER (12 INCH MINIMUM).

THE CONTRACTOR SHALL NOT REASSIGN THE CABINET DETECTOR INPUTS IN ORDER TO REDUCE THE NUMBER OF 2-CHANNEL DETECTOR UNITS SUPPLIED AND SHALL USE THE STANDARD CALTRANS INPUT FILE DESIGNATIONS FOLLOWING SHEET T6.

PAYMENT FOR ITEM 633 CABINET, TYPE 332L, AS PER PLAN WILL BE AT THE CONTRACT BID PRICE PER EACH COMPLETE AND IN PLACE INCLUDING ALL CONNECTIONS TESTED AND ACCEPTED.

ITEM 809 - ATC CONTROLLER, AS PER PLAN (PROGRAM AND INSTALL ONLY)

ALL REQUIREMENTS OF SS 809 SHALL BE FOLLOWED, ALONG WITH THE ADDITIONAL DESCRIPTION AS STATED BELOW. THE ATC CONTROLLER WILL BE PROVIDED BY THE DISTRICT WITHOUT PROGRAMMING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROGRAMMING THE CONTROLLER TO THE PROPOSED CONDITIONS ACCORDING TO THE PLANS. ODOT WILL NOT BE RESPONSIBLE FOR THE PROGRAMMING.

PAYMENT SHALL BE MADE ONCE THE CONTROLLER IS PROGRAMMED, INSTALLED, TESTED, FUNCTIONING ACCORDING TO THE PLANS, AND SHALL INCLUDE INCLUDE ALL LABOR, EQUIPMENT, MATERIALS, AND INCIDENTALS TO COMPLETE WORK.

CONTROLLER ITEM, MISC.: TIMING AND COORDINATION

THIS ITEM SHALL INCLUDE THE WORK NECESSARY TO CHANGE THE SIGNAL TIMING AND NETWORK COORDINATION IN THE LOCAL CONTROLLER LOCATED AT THE IR 70 WB RAMPS AS INDICATED ON SHEET T11.

COORDINATION TIMING WILL BE IMPLEMENTED AT THE FOLLOWING FOUR (4) INTERSECTIONS: S.R. 149 & RECO DRIVE, I-70 EASTBOUND RAMPS & S.R. 149, I-70 WESTBOUND RAMPS & S.R. 149, AND S.R. 149 & BOND DRIVE/SCHOOL STREET.

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TRAFFIC SIGNAL GENERAL NOTES

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ITEM 633 - UNINTERRUPTIBLE POWER SUPPLY (UPS), 1000 WATT, AS PER PLAN

IN ADDITION TO THE REQUIREMENTS OF C&MS 633 AND 733, POLE ATTACHMENT HARDWARE WILL BE INCLUDED FOR POLE-MOUNTED CABINETS, AND A CABINET RISER (8-INCH MINIMUM) AND ANCHOR BOLTS WILL BE PROVIDED FOR BASE-MOUNTED CABINETS. BEFORE PERFORMING THE WORK, THE CONTRACTOR, THE DISTRICT TRAFFIC ENGINEER, AND THE PROJECT ENGINEER WILL PERFORM A SITE INSPECTION TO ESTABLISH THE LOCATION OF THE UPS CABINET AND FOUNDATION.

THE UPS CABINET SHALL INCLUDE A GENERATOR POWER PANEL WITH A HEAVY-DUTY POWER RELAY VERSUS THE LINE VOLTAGE GENERATOR SWITCH. THE GENERATOR INLET SHALL BE A RECESSED PANEL WITH A DOOR THAT IS FLUSH WITH THE EXTERNAL SIDE OF THE UPS CABINET. IT SHALL INCLUDE A RECESSED PLUG, AUTOMATIC TRANSFER SWITCH AND A DOOR THAT SECURELY CLOSES OVER THE POWER CORD.

THE CABINET SHALL HAVE A DOOR STOP MECHANISM AND THERMOSTATICALLY CONTROLLED FAN.

THE CABINET SHALL INCLUDE A BATTERY BALANCING DEVICE THAT REGULATES THE BATTERIES AND OPTIMIZES PERFORMANCE.

AFTER FOUR (4) HOURS OF BATTERY RUNTIME, THE SYSTEM SHALL BE PROGRAMMED TO SWITCH THE INTERSECTION FROM FULL OPERATION TO CONTROLLER AUTOMATIC FLASH OPERATION THROUGH THE MONITOR. THE CONTROLLER SHALL BE PROGRAMMED SO THAT FLASH OPERATION SHALL BEGIN ONCE THE INTERSECTION RUNS MINOR STREET GREEN (TYP. PH. 4 & 8), ALL-RED CLEARANCE, AND THEN FLASH OPERATION.

THE UPS OUTPUT NOTIFICATIONS FOR ON BATTERY, BATTERY 2-HOUR TIMER, AND LOW BATTERY SHALL BE WIRED INTO THE TRAFFIC SIGNAL CABINET BACK PANEL OR THROUGH THE CONTROLLER WITH A C11 TO PROVIDE SPECIAL STATUS ALARMS FOR EACH OUTPUT INTO THE SIGNAL CONTROLLER.

THIS ITEM SHALL INCLUDE A RED LED STATUS INDICATOR LAMP TO ALLOW MAINTENANCE PERSONNEL AND LAW ENFORCEMENT TO QUICKLY ASSESS WHETHER A TRAFFIC SIGNAL CABINET IS BEING POWERED BY A UPS. THE LED HOUSING SHALL BE NEMA 4X, IP65 OR IP66, RATED FOR OUTDOOR USE AND BE TAMPER/SHATTER RESISTANT. IT SHALL BE A DOMED ENCLOSURE CONTAINING A RED LENS WITH LED THAT IS VISIBLE FROM 100 FOOT MINIMUM. THE ENCLOSURE AND LED MODULE SHOULD BE PLACED ON THE SIDE OF THE UPS CABINET FACING TOWARDS THE MAINLINE ROADWAY AND SEALED FROM WATER INTRUSION. IT SHOULD BE WIRED USING MINIMUM 20GA STRANDED, INSULATED HOOKUP WIRE TO THE STATUS RELAY OUTPUTS OF THE UPS. THE WIRES SHALL BE TERMINATED BY LUGS AT THE DISPLAY END AND PERMANENTLY LABELED "BACKUP POWER STATUS DISPLAY," WITH WIRE POLARITY INDICATED. THE RED LED SHALL ONLY ILLUMINATE TO INDICATE THE CABINET IS OPERATING UNDER UPS BACKUP POWER (THE "BACKUP" OPERATING CONDITION). THIS ITEM INCLUDES PROGRAMMING THE UPS STATUS RELAY OUTPUTS TO PRODUCE THE LAMP STATUS DISPLAYS. THESE STATUS DISPLAYS WILL BE SOLID 100% DUTY CYCLE (NOT FLASHING) DISPLAYS. THE OPERATING VOLTAGE OF THE LED LAMP SHALL BE 120V AC UNLESS OTHERWISE INDICATED.

ITEM 809 - ADVANCE RADAR DETECTION, AS PER PLAN

THIS ITEM OF WORK SHALL CONSIST OF FURNISHING AND INSTALLING A WAVETRONIX SMARTSENSOR ADVANCE DETECTION UNTIL (MODEL SS-200E). THE DETECTION UNIT SHALL INCLUDE THE FOLLOWING:

1. POWER SHALL BE PROVIDED FROM THE TRAFFIC CABINET.
2. ALL REQUIRED INPUTS CARDS SHALL BE INCLUDED IN THE TRAFFIC CABINET AND SHALL BE COMPATIBLE WITH CALTRANS, NEMA TS1 AND NEMA TS2 DETECTOR RACKS. THE CARDS SHALL PROVIDE TRUE PRESENCE DETECTOR CALLS OR CONTACT CLOSURE TO THE TRAFFIC CONTROLLER.
3. THE UNIT SHALL BE MOUNTED DIRECTLY TO A POLE OR MAST ARM, AS RECOMMENDED BY THE MANUFACTURER. CABLE(S) SHALL BE PROVIDED AS REQUIRED AND RECOMMENDED BY THE MANUFACTURER.
4. SURGE PROTECTION DEVICES, AS RECOMMENDED BY THE MANUFACTURER SHALL BE INCLUDED BOTH AT THE POLE WHERE THE UNIT IS LOCATED TO PROTECT THE UNIT AND IN THE TRAFFIC CABINET TO PROTECT THE CABINET ELECTRONICS.
5. THE MANUFACTURER'S REPRESENTATIVE SHALL BE ON SITE DURING INSTALLATION AND TESTING AND SHALL PROVIDE ONSITE TRAINING ON THE SETUP, OPERATION AND MAINTENANCE OF THE UNIT.
6. A SERIAL TO ETHERNET COMMUNICATIONS MODULE AND ETHERNET CABLE (MINIMUM 7 FEET).
7. THE POWER SUPPLY AND COMMUNICATION MODULES SHALL BE SECURED TO A SINGLE PANEL THAT CAN BE MOUNTED INTERIOR TO THE TRAFFIC CABINET. THE PANEL SHALL INCLUDE MODULAR-PLUG STYLE CONNECTIONS FOR UP TO FOUR (4) SENSOR CABLES. ADDITIONAL SENSORS MAY BE HARD-WIRED TO THE COMMUNICATION MODULES, AS NECESSARY.
8. THE CONTRACTOR SHALL INSTALL THE RADAR DETECTION PRIOR TO MILLING/DISABLING EXISTING LOOPS.
9. THE INSTALLATION SHALL INCLUDE ALL CONTROLLER PROGRAMMING FOR COMPLETE INSTALLATION, WHICH INCLUDES MODIFICATIONS FOR REMOVAL OF EXISTING DETECTION.

PAYMENT FOR ITEM 809 ADVANCE RADAR DETECTION, AS PER PLAN SHALL BE MADE AT THE CONTRACT UNIT PRICE FOR EACH UNIT, COMPLETE AND IN PLACE INCLUDING ALL REQUIRED CABINET HARDWARE, MOUNTING BRACKETS, CABLES, CONDUIT, CONNECTIONS TESTED AND ACCEPTED, AND ANY OTHER NECESSARY HARDWARE TO ESTABLISH A FULLY FUNCTIONAL DETECTION SYSTEM.

DETECTION MAINTENANCE

IF VEHICLE DETECTION BECOMES UNEXPECTEDLY DISABLED, REQUIRES MODIFICATION, OR IS SCHEDULED TO BE TEMPORARILY REMOVED DURING THE CONSTRUCTION PROJECT, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER.

IF THE LOSS OF VEHICLE DETECTION IS KNOWN PRIOR TO THE START OF CONSTRUCTION, IT SHALL BE DISCUSSED AT THE PRECONSTRUCTION MEETING. AT SUCH TIME, THE DISTRICT TRAFFIC ENGINEER SHALL ADVISE THE PROJECT ENGINEER AND CONTRACTOR ON THE APPROPRIATE ACTION TO RECTIFY ANY LOSS OF VEHICLE DETECTION. THIS MAY INCLUDE PLACING THE TRAFFIC SIGNAL ON MINIMUM OR MAXIMUM RECALL, MODIFYING THE MINIMUM GREEN TIMES, AND REMOVING THE MALFUNCTIONING DETECTION FROM SERVICE. WHERE NON-INTRUSIVE DETECTION (I.E. VIDEO, RADAR) ALREADY EXISTS, THE CONTRACTOR SHALL INSURE THAT DETECTION IS OPERATING AND MAINTAINED BY RECONFIGURING THE DETECTION UNITS ACCORDINGLY DURING ALL CONSTRUCTION PHASES. THIS IS TO AVOID THE SIGNAL FROM MAXING OUT THE EFFECTED SIGNAL PHASE AND CREATING UNNECESSARY DELAYS.

LOCATIONS WHERE NON-INTRUSIVE DETECTION IS PROPOSED AND THE EXISTING VEHICLE DETECTION IS TO BE ABANDON, THE NON-INTRUSIVE VEHICLE DETECTION SHALL BE INSTALLED, CONFIGURED AND MADE FULLY FUNCTIONAL PRIOR TO THE EXISTING DETECTION BEING DISABLED. THE CONTRACTOR SHALL CONTINUE TO MAINTAIN AND MODIFY THE DETECTION UNTIL FINAL ACCEPTANCE OF THE TRAFFIC SIGNAL. THIS IS TO ENSURE VEHICLE DETECTION REMAINS FULLY FUNCTIONAL THROUGHOUT CONSTRUCTION.

ITEM 633 - COMMUNICATIONS, AS PER PLAN

FURNISH A CELLULAR MODEM, ONE 3-ANTENNA ASSEMBLY (PART #6001136), AND A 10' ETHERNET CABLE FOR REMOTE WIRELESS CELLULAR COMMUNICATION.

FOR NETWORK CONSISTENCY CELLULAR MODEMS SHALL BE THE SIERRA WIRELESS:

MODEM, AIRLINK MP70 ETHERNET WITH AC TO DC POWER CABLE - MODEL 1102709KIT

THIS ITEM SHALL INCLUDE THE FURNISHING A MOUNTING BRACKET FOR THE ANTENNA WITH ALL NECESSARY HARDWARE INCLUDING BUT NOT LIMITED TO SPRING NUTS, WASHERS, AND BOLTS THAT INSTALLS TO THE MOUNTING CHANNEL ON THE SIDE OF THE SIGNAL CABINET.

THE CELLULAR MODEM EQUIPMENT SHALL BE DELIVERED TO ODOT DISTRICT 11 TRAFFIC FOR PROGRAMMING AND INSTALLATION.

JOSEPH PARISI, P.E.
(330) 308-7813
2201 REISER AVENUE SE
NEW PHILADELPHIA, OHIO 44663

THE CONTRACTOR SHALL PROVIDE THE MODEM SERIAL NUMBERS AND NECESSARY ESN NUMBERS FOR ODOT TO ESTABLISH WIRELESS SERVICE.

THE DEPARTMENT WILL MEASURE "COMMUNICATIONS, AS PER PLAN" BY THE NUMBER OF COMPLETE UNITS FURNISHED, RECEIVED, AND ACCEPTED BY ODOT DISTRICT 11 TRAFFIC.

ITEM 809 - STOP LINE RADAR DETECTION, AS PER PLAN

THIS ITEM OF WORK SHALL CONSIST OF FURNISHING AND INSTALLING A WAVETRONIX SMARTSENSOR MATRIX DETECTION UNIT. THE DETECTION UNIT SHALL INCLUDE THE FOLLOWING:

1. POWER SHALL BE PROVIDED FROM THE TRAFFIC CABINET.
2. ALL REQUIRED INPUTS CARDS SHALL BE INCLUDED IN THE TRAFFIC CABINET AND SHALL BE COMPATIBLE WITH CALTRANS, NEMA TS1 AND NEMA TS2 DETECTOR RACKS. THE CARDS SHALL PROVIDE TRUE PRESENCE DETECTOR CALLS OR CONTACT CLOSURE TO THE TRAFFIC CONTROLLER.
3. THE UNIT SHALL BE MOUNTED DIRECTLY TO A POLE OR MAST ARM, AS RECOMMENDED BY THE MANUFACTURER. CABLE(S) SHALL BE PROVIDED AS REQUIRED AND RECOMMENDED BY THE MANUFACTURER.
4. SURGE PROTECTION DEVICES, AS RECOMMENDED BY THE MANUFACTURER SHALL BE INCLUDED BOTH AT THE POLE WHERE THE UNIT IS LOCATED TO PROTECT THE UNIT AND IN THE TRAFFIC CABINET TO PROTECT THE CABINET ELECTRONICS.
5. THE MANUFACTURER'S REPRESENTATIVE SHALL BE ON SITE DURING INSTALLATION AND TESTING AND SHALL PROVIDE ONSITE TRAINING ON THE SETUP, OPERATION AND MAINTENANCE OF THE UNIT.
6. A SERIAL TO ETHERNET COMMUNICATIONS MODULE AND ETHERNET CABLE (MINIMUM 7 FEET).
7. THE POWER SUPPLY AND COMMUNICATION MODULES SHALL BE SECURED TO A SINGLE PANEL THAT CAN BE MOUNTED INTERIOR TO THE TRAFFIC CABINET. THE PANEL SHALL INCLUDE MODULAR-PLUG STYLE CONNECTIONS FOR UP TO FOUR (4) SENSOR CABLES. ADDITIONAL SENSORS MAY BE HARD-WIRED TO THE COMMUNICATION MODULES, AS NECESSARY.
8. THE CONTRACTOR SHALL INSTALL THE RADAR DETECTION PRIOR TO MILLING/DISABLING EXISTING LOOPS.
9. THE INSTALLATION SHALL INCLUDE ALL CONTROLLER PROGRAMMING FOR COMPLETE INSTALLATION, WHICH INCLUDES MODIFICATIONS FOR REMOVAL OF EXISTING DETECTION.

PAYMENT FOR ITEM 809 STOP-LINE RADAR DETECTION, AS PER PLAN SHALL BE MADE AT THE CONTRACT UNIT PRICE FOR EACH UNIT, COMPLETE AND IN PLACE INCLUDING ALL REQUIRED CABINET HARDWARE, MOUNTING BRACKETS, CABLES, CONDUIT AND CONNECTIONS TESTED AND ACCEPTED.

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GROUNDING AND BONDING

THE REQUIREMENTS OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS (C&MS) AND THE TC SERIES OF STANDARD CONSTRUCTION DRAWINGS ARE MODIFIED AS FOLLOWS:

1. ALL METALLIC PARTS CONTAINING ELECTRICAL CONDUCTORS SHALL BE PERMANENTLY JOINED TO FORM AN EFFECTIVE GROUND FAULT CURRENT PATH BACK TO THE GROUNDED CONDUCTOR IN THE POWER SERVICE DISCONNECT SWITCH.
 - A. PROVIDE AN EQUIPMENT GROUNDING CONDUCTOR IN METALLIC CONDUITS (725.04) IN ADDITION TO THE CONDUCTORS SPECIFIED AND BOND THE CONDUIT TO THIS GROUNDING CONDUCTOR.
 - B. WHEN AN EQUIPMENT GROUNDING CONDUCTOR IS REQUIRED IN PLASTIC CONDUIT (725.05), THE INSTALLATION SHALL INCLUDE A SEPARATE EQUIPMENT GROUNDING CONDUCTOR IN ADDITION TO THE CONDUCTORS SPECIFIED.
 - C. METALLIC CONDUIT CARRYING THE LOOP WIRES FROM IN THE PAVEMENT TO THE PULL BOX SPLICE LOCATION WILL ONLY BE BONDED AT THE PULL BOX END, AND WILL NOT CONTAIN AN EQUIPMENT GROUNDING CONDUCTOR.
 - D. IF MULTIPLE CONDUIT RUNS BEGIN AND END AT THE SAME POINTS, ONLY ONE EQUIPMENT GROUNDING CONDUCTOR IS REQUIRED.
 - E. IF AN EQUIPMENT GROUNDING CONDUCTOR IS NEEDED IN CONDUIT BETWEEN SIGNALIZED INTERSECTIONS FOR UNDERGROUND INTERCONNECT CABLE, THE GROUNDING SYSTEM FOR EACH SIGNALIZED INTERSECTION WILL BE SEPARATED ABOUT MIDWAY BETWEEN THE INTERSECTIONS.
 - F. THE MESSENGER WIRE AT SIGNALIZED INTERSECTIONS WILL BE USED AS THE CONDUCTIVE PATH FROM CORNER TO CORNER IF CONDUIT IS NOT PROVIDED UNDER THE ROADWAY. WHEN CONDUIT CONNECTS THE CORNERS OF AN INTERSECTION, AN EQUIPMENT GROUNDING CONDUCTOR SHALL BE USED IN THE CONDUIT.

2. CONDUITS

- A. THE 725.04 CONDUIT SHALL HAVE GROUNDING BUSHINGS INSTALLED AT ALL TERMINATION POINTS. THE BUSHING MATERIAL SHALL BE COMPATIBLE WITH GALVANIZED STEEL CONDUIT AND THE GROUNDING LUG MATERIAL SHALL BE COMPATIBLE FOR USE WITH COPPER WIRE. THREADED OR COMPRESSION TYPE BUSHINGS MAY BE USED.
- B. THE 725.05 CONDUIT SHALL HAVE THE INSIDE AND OUTSIDE DIAMETERS OF THE CONDUIT DEBURRED AT ALL TERMINATION POINTS.
- C. BOTH ENDS OF METALLIC CONDUIT SHALL BE BONDED TO THE EQUIPMENT GROUNDING CONDUCTOR.
- D. METALLIC CONDUIT MAY BE BONDED TO METALLIC BOXES THROUGH THE USE OF CONDUIT FITTINGS UL APPROVED FOR THIS TYPE OF CONNECTION, WITH THE BOX BONDED TO THE EQUIPMENT GROUNDING CONDUCTOR.

GROUNDING AND BONDING (CONT.)

3. WIRE FOR GROUNDING AND BONDING.

- A. USE INSULATED, COPPER WIRE FOR THE EQUIPMENT GROUNDING CONDUCTOR. BONDING JUMPERS IN BOXES AND ENCLOSURES MAY BE BARE OR INSULATED COPPER WIRE. WIRE SIZE SHALL BE AS FOLLOWS:
 - I. USE 4 AWG BETWEEN THE POWER SERVICE AND SUPPORTS, POLES, PEDESTALS, CONTROLLER OR FLASHER CABINETS.
 - II. USE A MINIMUM 8 AWG BETWEEN LOOP DETECTOR PULL BOXES AND THE FIRST CONDUIT THAT REQUIRES A LARGER SIZE AS SPECIFIED IN 3.A.I ABOVE.
 - III. USE A MINIMUM 8 AWG BETWEEN THE "PREPARE TO STOP WHEN FLASHING" INSTALLATION (INCLUDING SUPPORT) AND THE FIRST CONDUIT THAT REQUIRES A LARGER SIZE AS SPECIFIED IN 3.A.I ABOVE.
 - IV. THE INSULATION SHALL BE GREEN OR GREEN WITH YELLOW STRIPE(S). FOR 4 AWG OR LARGER, INSULATION MAY ALSO BE BLACK WITH GREEN TAPE/LABELS INSTALLED AT ALL ACCESS POINTS.
- B. IN A HIGHWAY LIGHTING SYSTEM, THE EQUIPMENT GROUNDING CONDUCTOR SHALL BE THE SAME WIRE SIZE AS THE DUCT CABLE OR DISTRIBUTION CABLE CIRCUIT CONDUCTORS, WITH THE MINIMUM CONDUCTOR SIZE OF 4 AWG. BONDING JUMPERS WILL BE MINIMUM SIZE 4 AWG.

4. GROUND ROD

- A. A 3/4 INCH SCHEDULE 40 PVC CONDUIT WILL BE USED IN FOUNDATIONS AND CONCRETE WALLS FOR THE GROUNDING CONDUCTOR (GROUND WIRE) RACEWAY TO THE GROUND ROD. SHOULD METALLIC CONDUIT BE USED, BOTH ENDS OF THE CONDUIT SHALL BE BONDED TO THE GROUNDING CONDUCTOR.
- B. THE TYPICAL GROUNDING CONDUCTOR (GROUND WIRE) SHALL BE 4 AWG INSULATED, COPPER.

5. THE GREEN CONDUCTOR IN SIGNAL CABLES (CONDUCTOR #4) SHALL NOT BE USED TO SUPPLY POWER TO A SIGNAL INDICATION. IT WILL BE CONNECTED TO THE SIGNAL BODY AS AN EQUIPMENT GROUND IN ALUMINUM HEADS AND IT WILL BE UNUSED IN PLASTIC HEADS. UNUSED CONDUCTORS SHALL BE GROUNDED IN THE CABINET. TYPICAL USE OF CONDUCTORS IS AS FOLLOWS:

COND. NO	COLOR	VEHICLE SIGNAL	PEDESTRIAN SIGNAL
1	BLACK	GREEN BALL	#1 WALK
2	WHITE	AC NEUTRAL	AC NEUTRAL
3	RED	RED BALL	#1 DW/FDW
4	GREEN	EQUIPMENT GROUND	EQUIPMENT GROUND
5	ORANGE	YELLOW BALL	#2 DW/FDW
6	BLUE	GREEN ARROW	#2 WALK
7	WHITE/BLACK STRIPE	YELLOW ARROW	NOT USED

GROUNDING AND BONDING (CONT.)

6. POWER SERVICE AND DISCONNECT SWITCH.

- A. AT THE POWER SERVICE LOCATION, THE GROUNDING CONDUCTOR (GROUND WIRE) FROM THE DISCONNECT SWITCH NEUTRAL (AC-) BAR TO THE GROUND ROD SHALL BE A CONTINUOUS, UNSPLICED CONDUCTOR. IF SPLICED, IT SHALL BE AN EXOTHERMIC WELD BUTT SPLICE.
 - B. THE SERVICE NEUTRAL (AC-) SHALL ONLY BE CONNECTED TO GROUND AT THE PRIMARY POWER SERVICE DISCONNECT SWITCH.
 - I. NEMA CONTROLLER CABINETS: IF A POWER SERVICE DISCONNECT SWITCH IS LOCATED BEFORE THE CONTROLLER CABINET, THE NEUTRAL (AC-) AND THE GROUNDING BARS IN THE CONTROLLER CABINET SHALL NOT BE CONNECTED TOGETHER AS SHOWN IN NEMA TS-2, FIGURE 5-4.
 - II. IF SECONDARY DISCONNECT SWITCHES ARE CONNECTED AFTER THE PRIMARY DISCONNECT SWITCH, THE NEUTRAL (AC-) SHALL ONLY BE GROUNDED AT THE PRIMARY SWITCH. EQUIPMENT GROUNDING CONDUCTORS SHALL BE BROUGHT TO THE PRIMARY SWITCH, BUT SHALL BE GROUNDED AT BOTH SECONDARY AND PRIMARY SWITCHES.
7. PAYMENT - ALL MATERIALS AND WORK REQUIRED TO COMPLETE THE EFFECTIVE GROUND FAULT CURRENT PATH SYSTEM ARE INCIDENTAL TO THE CONDUCTORS INSTALLED BY CONTRACT.

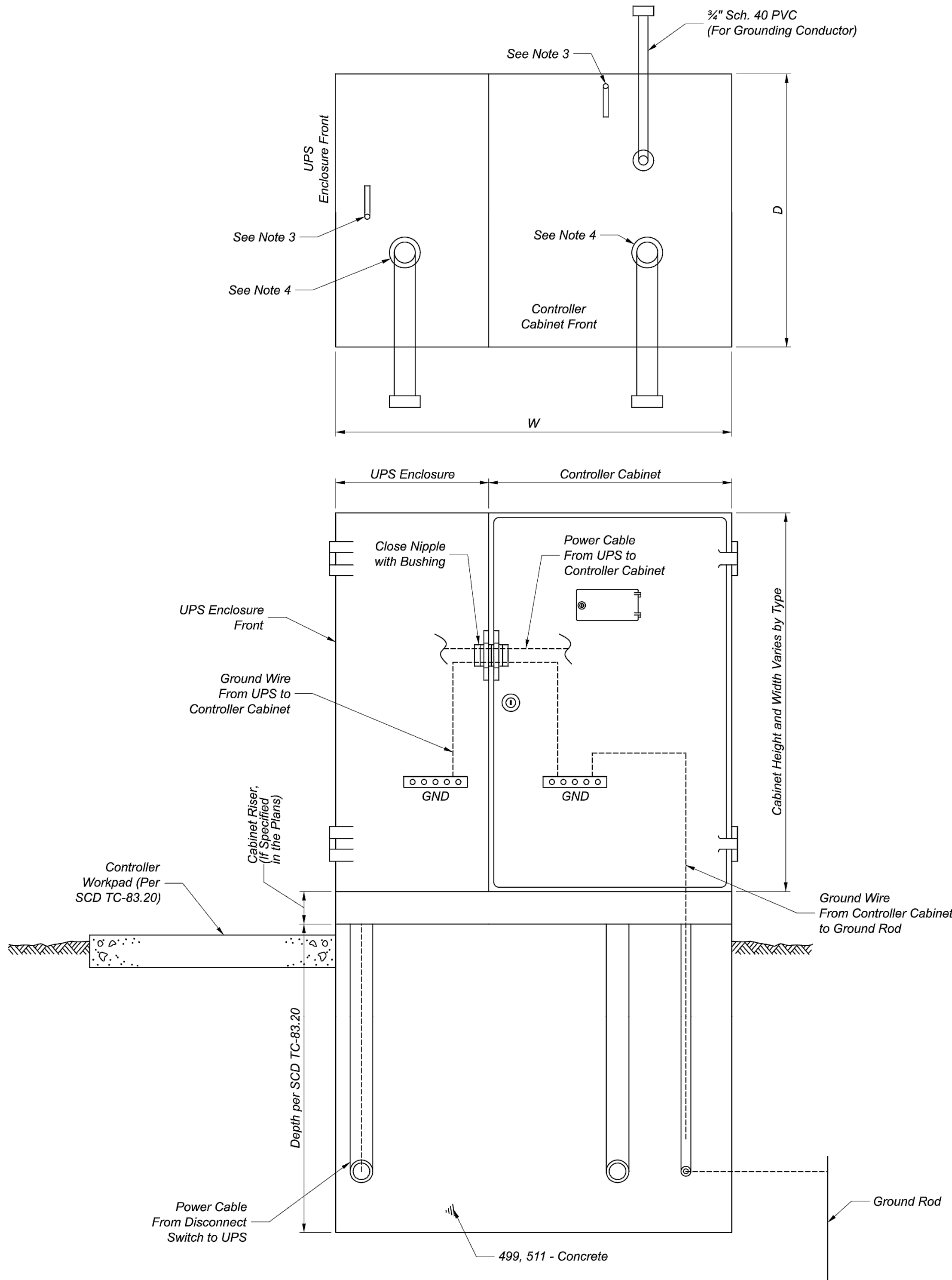
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NOTES:

1. THE UNINTERRUPTIBLE POWER SUPPLY (UPS) ENCLOSURE SHALL BE MOUNTED FLUSH UP AGAINST THE TRAFFIC SIGNAL CABINET AND SEALED WITH SILICONE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE NECESSARY POWER CABLE BETWEEN THE UPS UNIT AND SIGNAL CABINET.
2. THE UPS SHOULD BE PLACED ON THE OPPOSITE SIDE OF THE PULL BOX ON A 332/336 CABINET (PER STANDARD CONSTRUCTION DRAWING (SCD) TC-83.20). THE UPS PLACEMENT FOR A NEMA CABINET VARIES, PLACEMENT SHOULD PROVIDE ADEQUATE ACCESS WITH RESPECT TO SLOPE, GUARDRAIL SPACING, ETC.
3. THE SIZE, NUMBER, AND LOCATION OF ANCHOR BOLTS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
4. THE SIZE, NUMBER, AND ORIENTATION OF CONDUIT ELLS SHALL BE AS SHOWN IN THE PLAN, EXCEPT THAT A 3/4\"/>

TYPE	W (IN.)	D (IN.)	FOUNDATION CONCRETE (CU. YD.)
TS-1	60	24	1.23
TS-2	70	36	2.16
2070/170	50	36	1.54

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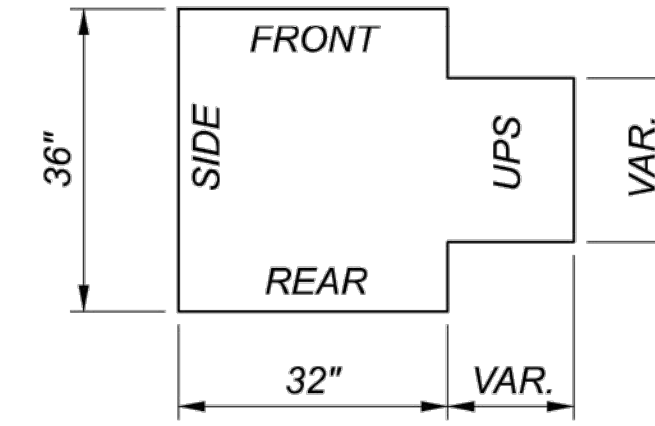
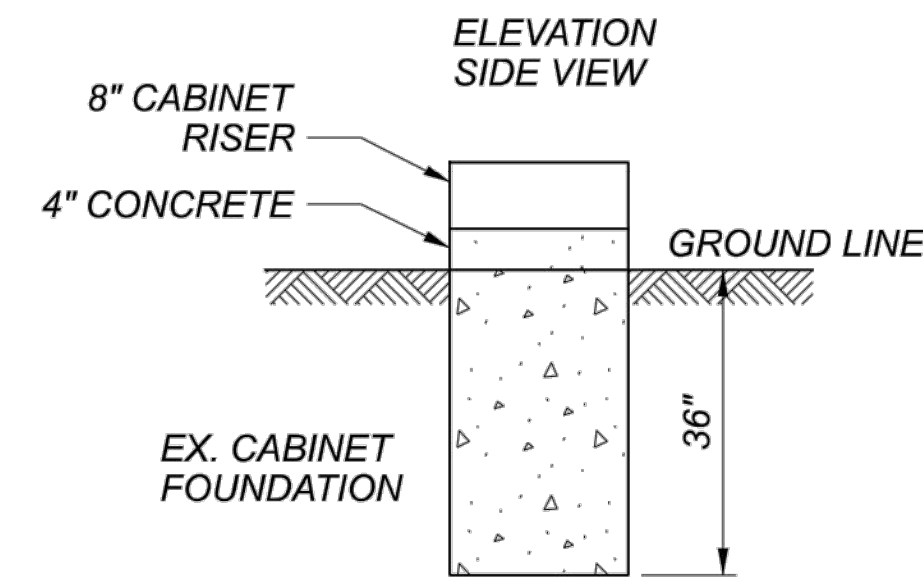
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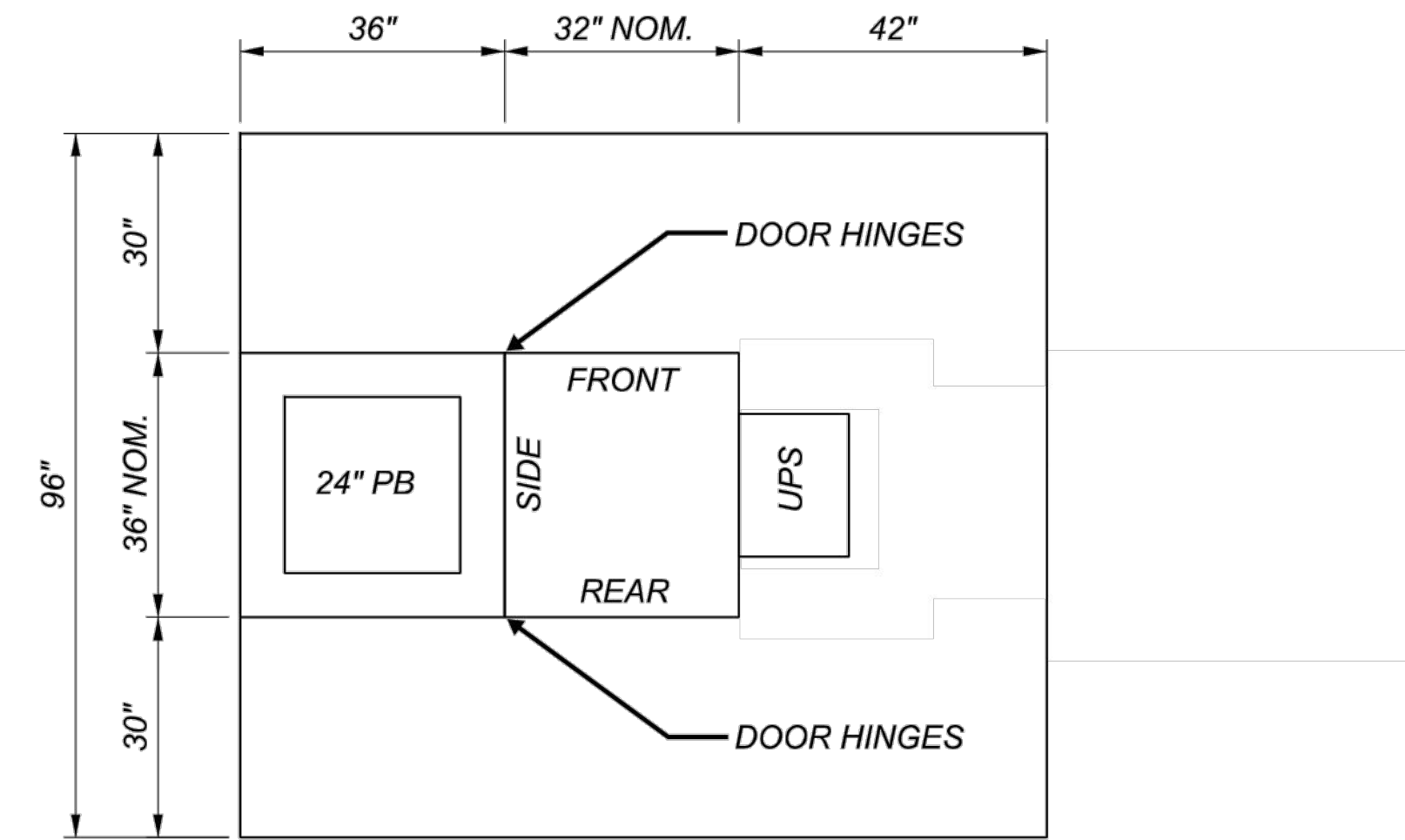
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MODEL 332 CABINET DETAIL (TYP.)

UPS FOUNDATION DETAIL



UPS WORK PAD DETAIL



PLAN VIEW

NOTES:

- 1) THE SIZE OF THE UPS FOUNDATION MAY VARY BASED ON THE CABINET SIZE PROVIDED.
- 2) UPS FOUNDATION ELEVATION SHOULD MATCH CABINET FOUNDATION ELEVATION.
- 3) THE UPS CABINET SHALL BE MOUNTED FLUSH UP AGAINST THE SIGNAL CABINET AND SEALED.
- 4) CONDUIT AND WIRING FROM THE SIGNAL CABINET TO THE UPS SHALL BE INSTALLED THROUGH THE CABINET RISER.

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SEPAC AND ASC/3 INPUT FILE INFORMATION FOR THE 332 CABINET

UPPER INPUT FILE (FILE=I)

C U H P A P N E N R E L	PHASE	1	2	2	2	3	4	4	4	1		MANUAL CONTROL ADV.	2	6	FLASH
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	SPARE		PED	PED	SENSE
	SEPAC DETECTOR NO.	VEH 1	VEH 3	VEH 5	VEH 7	VEH 9	VEH 11	VEH 13	VEH 15	VEH 17			PED 2	PED 6	
	ASC/3 DETECTOR NO.	VEH 1	VEH 2	VEH 3	VEH 4	VEH 5	VEH 6	VEH 7	VEH 8	VEH 9			PED 2	PED 6	
	C1 PIN NUMBER	56	39	63	47	58	41	65	49	60		80	67	68	81
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E	
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14
C L H O A W N E N R E L	PHASE	1	2	2	2	3	4	4	4	3		ADV.	4	8	STOP
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	SPARE	ENABLE	PED	PED	TIME
	SEPAC DETECTOR NO.	VEH 1	VEH 4	VEH 6	VEH 7	VEH 9	VEH 12	VEH 14	VEH 15	VEH 18			PED 4	PED 8	
	ASC/3 DETECTOR NO.	VEH 1	VEH 10	VEH 11	VEH 4	VEH 5	VEH 14	VEH 15	VEH 8	VEH 13			PED 4	PED 8	
	C1 PIN NUMBER	56	43	76	47	58	45	78	49	62		53	69	70	82
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K	

LOWER INPUT FILE (FILE=J)

C U H P A P N E N R E L	PHASE	5	6	6	6	7	8	8	8	5		SPARE	SPARE	EV - A	EV - B	RR - 1
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	SPARE					
	SEPAC DETECTOR NO.	VEH 19	VEH 21	VEH 23	VEH 25	VEH 29	VEH 31	VEH 33	VEH 35	VEH 37						
	ASC/3 DETECTOR NO.	VEH 17	VEH 18	VEH 19	VEH 20	VEH 21	VEH 22	VEH 23	VEH 24	VEH 25						
	C1 PIN NUMBER	55	40	64	48	57	42	66	50	59		54	71	72	51	
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E		
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14	
C L H O A W N E N R E L	PHASE	5	6	6	6	7	8	8	8	7		SPARE	SPARE	EV - C	EV - D	RR - 2
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	SPARE					
	SEPAC DETECTOR NO.	VEH 19	VEH 22	VEH 24	VEH 25	VEH 29	VEH 32	VEH 34	VEH 35	VEH 38						
	ASC/3 DETECTOR NO.	VEH 17	VEH 26	VEH 27	VEH 20	VEH 21	VEH 30	VEH 31	VEH 24	VEH 29						
	C1 PIN NUMBER	55	44	77	48	57	46	79	50	61		75	73	74	52	
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K		

SEPAC AND ASC/3 INPUT FILE INFORMATION FOR THE 336 CABINET

C U H P A P N E N R E L	PHASE	1	2	3	4	5	6	7	8				2	6	FLASH
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	RR - 1	EV - A	EV - B	PED	PED	SENSE
	SEPAC DETECTOR NO.	VEH 1	VEH 3	VEH 9	VEH 11	VEH 19	VEH 21	VEH 29	VEH 31				PED 2	PED 6	
	ASC/3 DETECTOR NO.	VEH 1	VEH 2	VEH 5	VEH 6	VEH 17	VEH 18	VEH 21	VEH 22				PED 2	PED 6	
	C1 PIN NUMBER	56	39	58	41	55	40	57	42	51	71	72	67	68	81
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E	
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14
C L H O A W N E N R E L	PHASE	2	2	4	4	6	6	8	8				4	8	STOP
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	RR - 2	EV - C	EV - D	PED	PED	TIME
	SEPAC DETECTOR NO.	VEH 7	VEH 4	VEH 15	VEH 12	VEH 25	VEH 22	VEH 35	VEH 32				PED 4	PED 8	
	ASC/3 DETECTOR NO.	VEH 4	VEH 10	VEH 8	VEH 14	VEH 20	VEH 26	VEH 24	VEH 30				PED 4	PED 8	
	C1 PIN NUMBER	47	43	49	45	48	44	50	46	52	73	74	69	70	82
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K	

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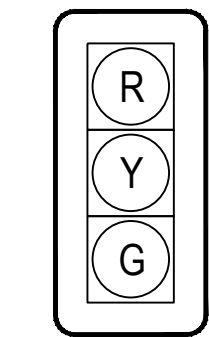
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TRAFFIC SIGNAL GENERAL NOTES
S.R. 149

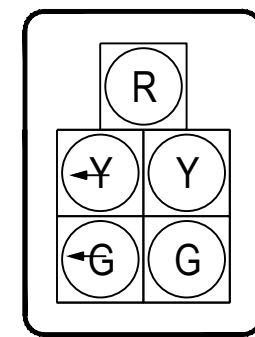
BEL-149-23.44

T6
T14

PROPOSED VEHICULAR SIGNAL HEADS



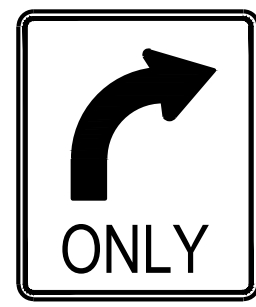
SIGNALS
2A 2B 4A
4B 6A 6B



SIGNALS
5A

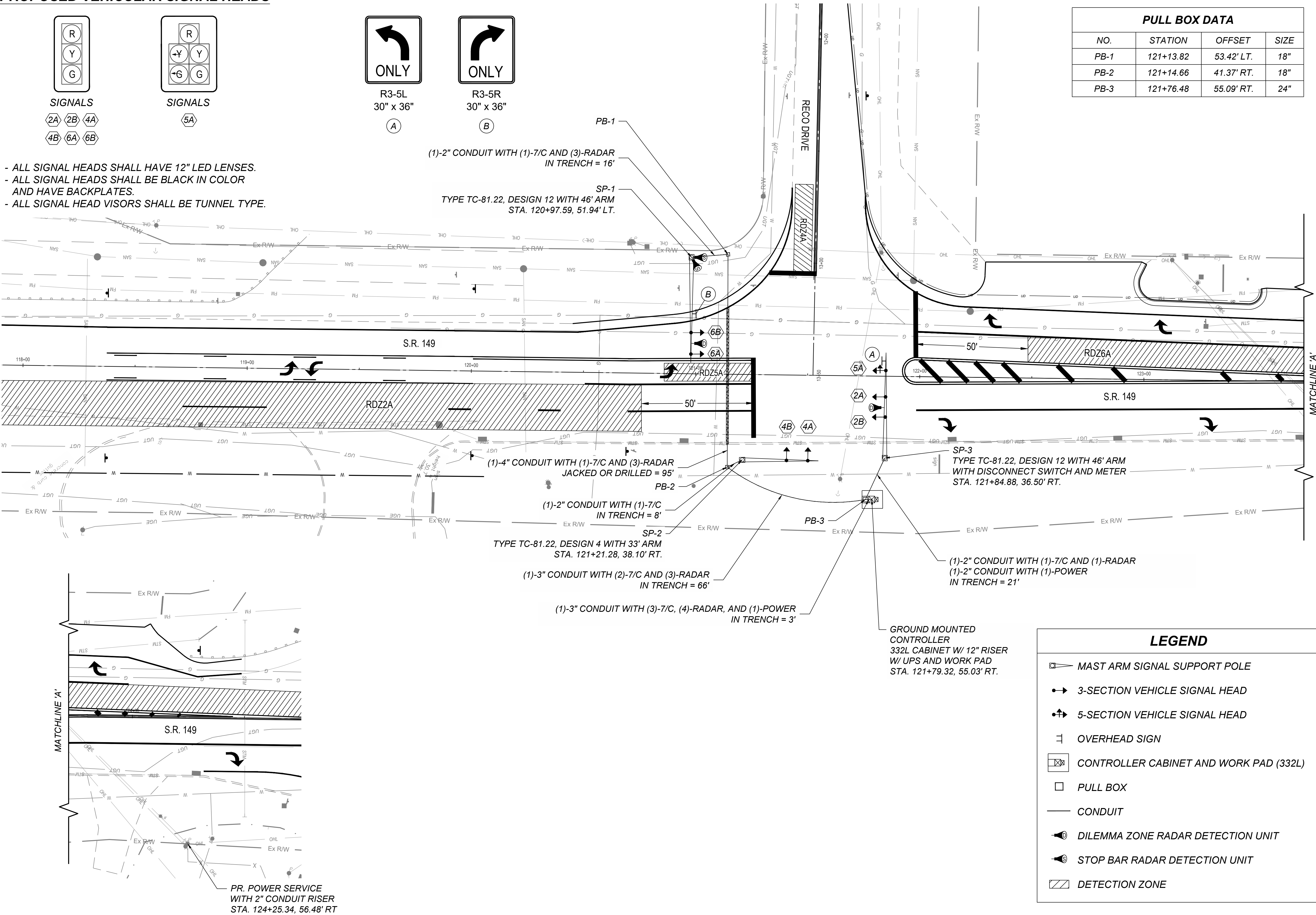


R3-5L
30" x 36"
A

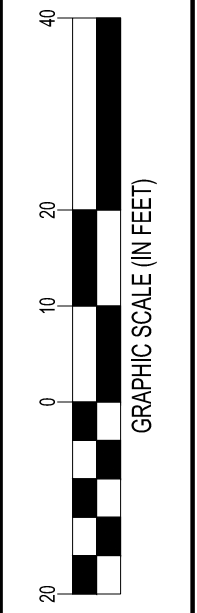
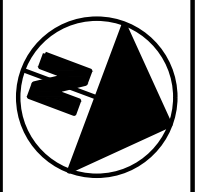


R3-5R
30" x 36"
B

- ALL SIGNAL HEADS SHALL HAVE 12" LED LENSES.
- ALL SIGNAL HEADS SHALL BE BLACK IN COLOR AND HAVE BACKPLATES.
- ALL SIGNAL HEAD VISORS SHALL BE TUNNEL TYPE.



PULL BOX DATA			
NO.	STATION	OFFSET	SIZE
PB-1	121+13.82	53.42' LT.	18"
PB-2	121+14.66	41.37' RT.	18"
PB-3	121+76.48	55.09' RT.	24"



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TRAFFIC SIGNAL PLAN
S.R. 149

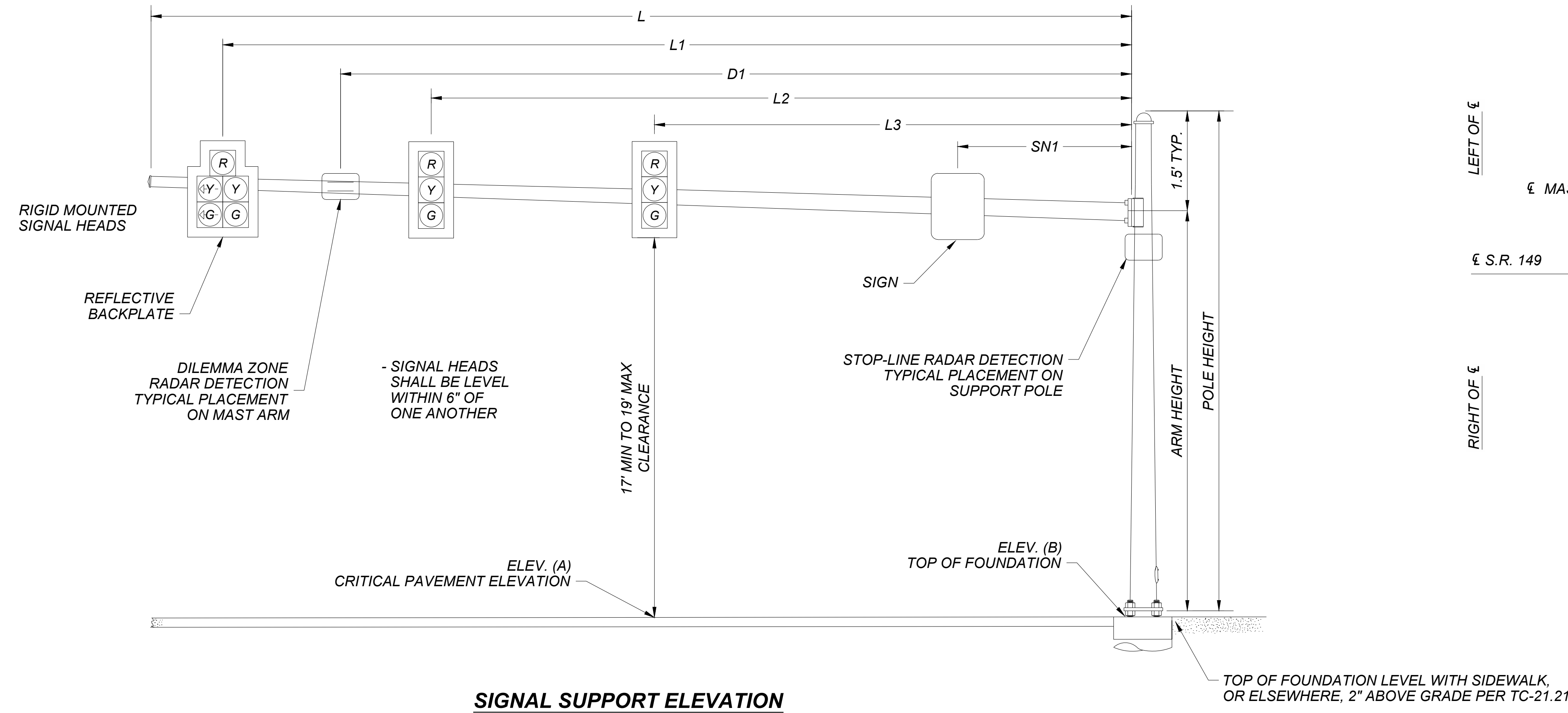
BEL-149-23.44

T7
T14

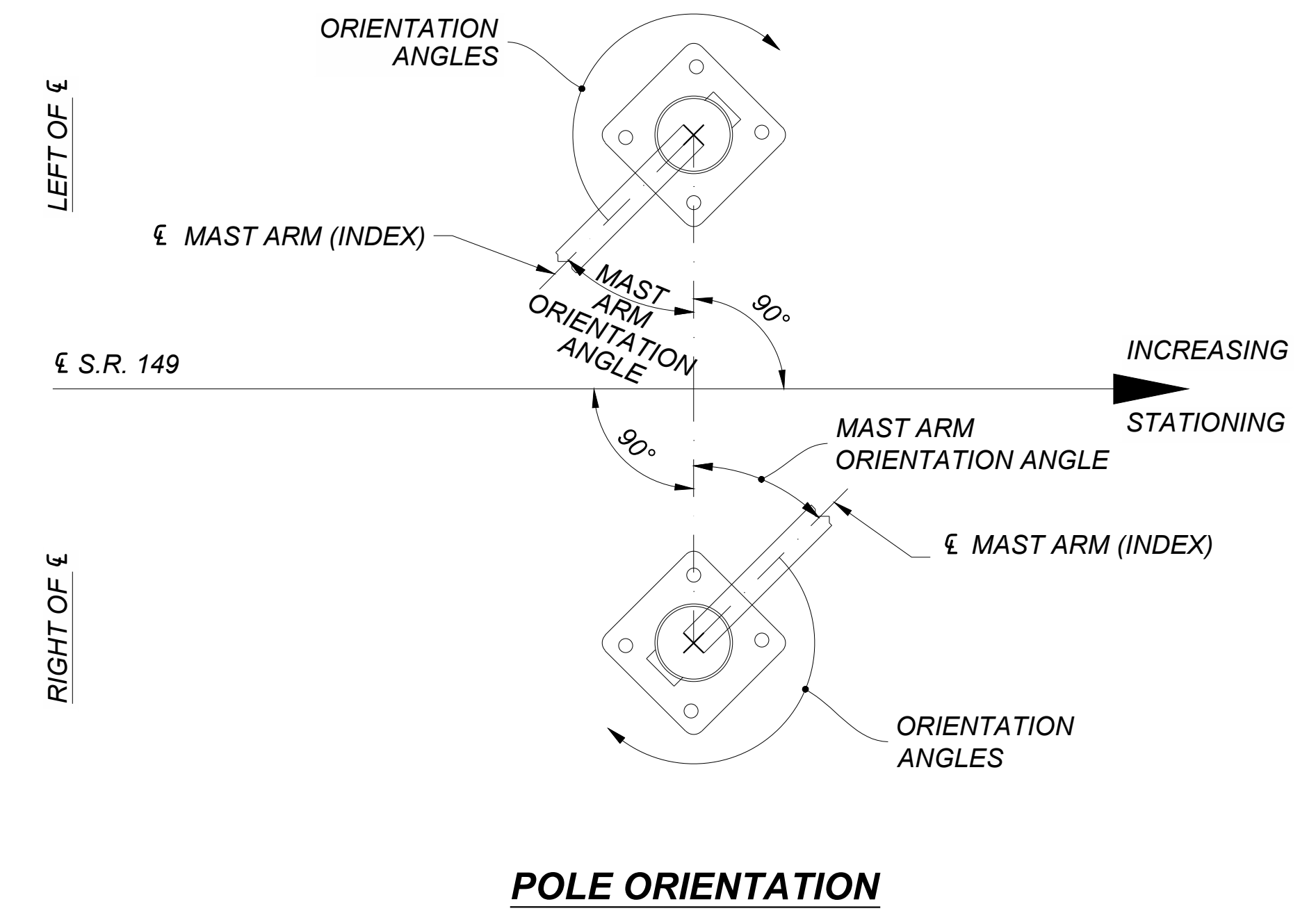
LEGEND	
	MAST ARM SIGNAL SUPPORT POLE
	3-SECTION VEHICLE SIGNAL HEAD
	5-SECTION VEHICLE SIGNAL HEAD
	OVERHEAD SIGN
	CONTROLLER CABINET AND WORK PAD (332L)
	PULL BOX
	CONDUIT
	DILEMMA ZONE RADAR DETECTION UNIT
	STOP BAR RADAR DETECTION UNIT
	DETECTION ZONE

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SIGNAL SUPPORT ELEVATION



POLE ORIENTATION

MAST ARM TABLE

TEM Fig. 498-37: Plan Details for Signal Supports - Arm Lengths																TEM Fig. 498-38: Plan Details for Signal Supports - Mast Arm Orientation				
SUPPORT NO.	STATION	OFFSET	ELEVATION		SIGNAL SUPPORT DETAILS												ORIENTATION ANGLES FROM MAST ARM			
			A (Pavt. Elev.)	B (Top of Found.)	DESIGN TYPE	DESIGN NO.	POLE HEIGHT	ARM HEIGHT	L	L1	L2	L3	R1	R2	R3	SN1	MAST ARM ANGLE	POWER SERVICE	HANDHOLE	CABLE ENTRANCE 12" FROM TOP
								FT	FT	FT	FT	FT	FT	FT	FT	FT	DEG	DEG	DEG	DEG
SP-1	120+97.59	51.94' LT	1191.07	1191.92	TC-81.22	12	21	19.5	46	42	33	-	0	0	37.5	24	0	-	180	-
SP-2	121+21.28	38.10' RT	1192.35	1192.26	TC-81.22	4	21	19.5	33	19	28.5	-	-	-	-	-	90	-	180	-
SP-3	121+84.88	36.50' RT	1191.88	1192.72	TC-81.22	12	21	19.5	46	17.5	26.5	38	21.5	-	-	42.5	0	180	180	180
-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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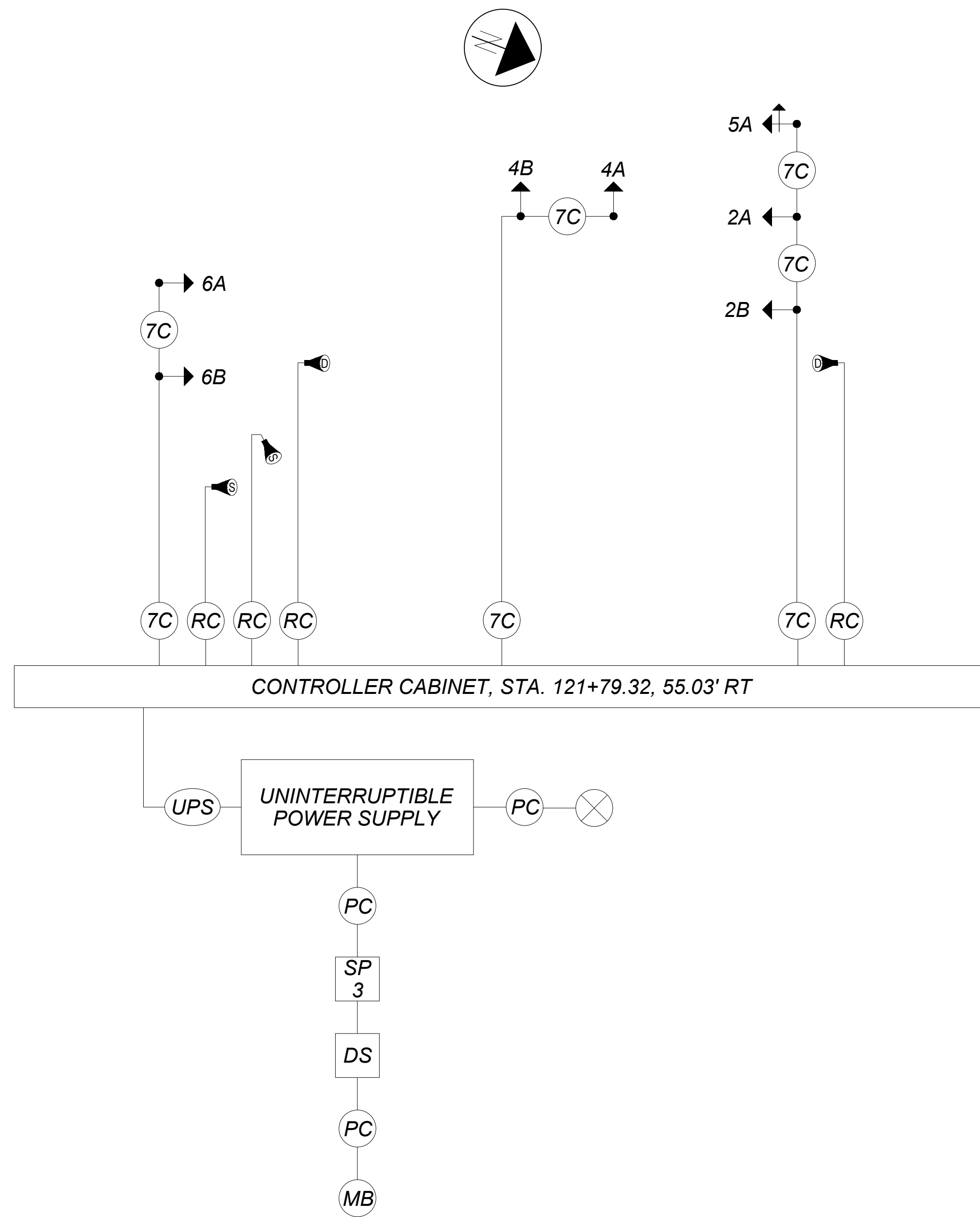
TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

BEL-149-23.44

T8
T14

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WIRING DIAGRAM



FIELD WIRING HOOK-UP CHART

SIGNAL HEAD	INDICATION	FIELD TERMINAL	FLASH
5A (NB LT)	R	Ø2 R	R
	Y	Ø2 Y	
	G	Ø2 G	
	<-Y--	Ø5 Y	
2A, 2B (NB)	<-G--	Ø5 G	R
	R	Ø2 R	
4A, 4B (EB)	Y	Ø2 Y	R
	G	Ø2 G	
6A, 6B (SB)	R	Ø4 R	R
	Y	Ø4 Y	
	G	Ø4 G	
LS = LOAD SWITCH			

WIRING DIAGRAM LEGEND

- 5-SECTION VEHICULAR SIGNAL HEAD, 1-WAY
- 3-SECTION VEHICULAR SIGNAL HEAD, 1-WAY
- STOP BAR RADAR DETECTION UNIT
- DILEMMA ZONE RADAR DETECTION UNIT
- SIGNAL CABLE, 7 CONDUCTOR, NO. 14 AWG
- RADAR DETECTION CABLE
- POWER SOURCE
- POWER CABLE, 2 CONDUCTOR, NO. 6 AWG
- METER BASE
- SIGNAL DISCONNECT SWITCH
- UNINTERRUPTIBLE POWER SUPPLY CABLE

TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

BEL-149-23.44

T9
T14

SIGNAL TIMING CHART

INTERSECTION: S.R. 149 & RECO DRIVE MAINTAINING AGENCY: ODOT DISTRICT 11									
START UP		DUAL ENTRY:	YES	PHASES:		2 + 6			
START IN:		ALL RED		RING 1		RING 2			
TIME FOR FLASH, ALL RED:		5 SEC.		OVERLAP		A	B	C	D
FIRST PHASE(S):		Ø2 + Ø6		PHASES		-	-	-	-
COLOR DISPLAYED:		GREEN							
INTERVAL OR FEATURE	CONTROLLER MOVEMENT NO.								
INTERSECTION MOVEMENT (PHASE)	1	2	3	4	5	6	7	8	
DIRECTION	-	NB	-	EB	NBLT	SB	-	-	
MINIMUM GREEN (INITIAL) (SEC.)	-	20	-	10	7	20	-	-	
ADDED INITIAL *(SEC./ACTUATION)	-	-	-	-	-	-	-	-	
MAXIMUM INITIAL *(SEC.)	-	-	-	-	-	-	-	-	
PASSAGE TIME (PRESET GAP) (SEC.)	-	2.0	-	2.0	2.0	2.0	-	-	
TIME BEFORE REDUCTION *(SEC.)	-	-	-	-	-	-	-	-	
MINIMUM GAP *(SEC.)	-	-	-	-	-	-	-	-	
TIME TO REDUCE *(SEC.)	-	-	-	-	-	-	-	-	
MAXIMUM GREEN I (SEC.)	-	34	-	14	8	21	-	-	
MAXIMUM GREEN II (SEC.)	-	-	-	-	-	-	-	-	
YELLOW CHANGE (SEC.)	-	4.5	-	4.5	4.0	5.0	-	-	
ALL RED CLEARANCE (SEC.)	-	1.0	-	1.0	1.5	1.0	-	-	
DELAYED GREEN (LPI) # (SEC.)	-	-	-	-	-	-	-	-	
WALK (SEC.)	-	-	-	-	-	-	-	-	
PEDESTRIAN CLEARANCE (SEC.)	-	-	-	-	-	-	-	-	
RECALL	MAXIMUM (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF
	MINIMUM (ON/OFF)	-	ON	-	OFF	-	ON	-	OFF
	PEDESTRIAN (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF
MEMORY (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF	

*VOLUME DENSITY CONTROLS

NOTES:

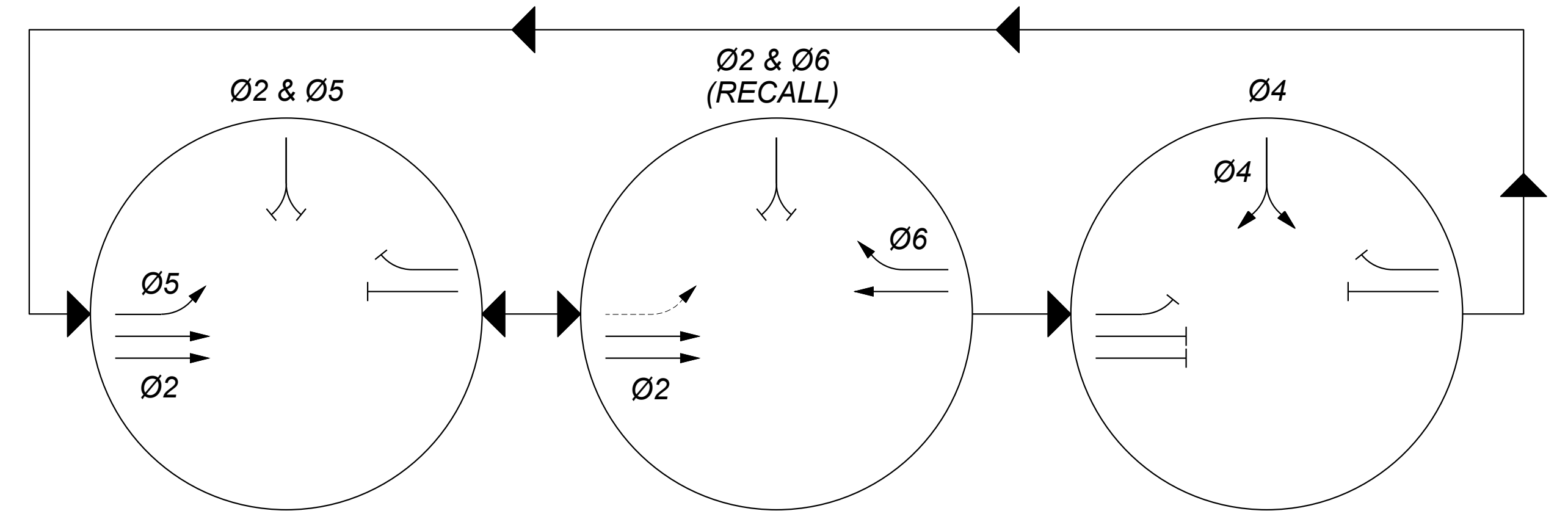
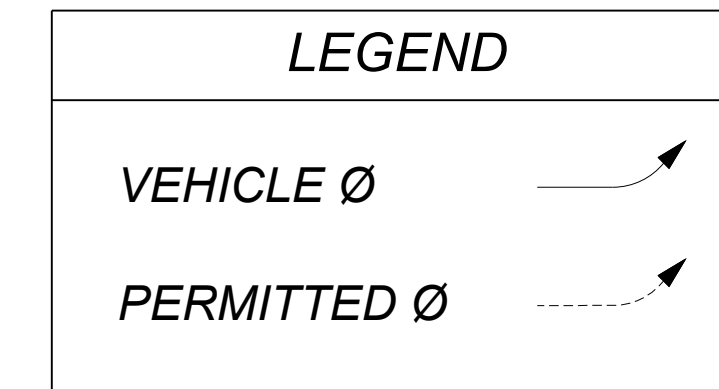
- ALL MOVEMENTS SHALL BE ACTUATED. THE PRIMARY THRU MOVEMENT SHOULD HAVE MIN RECALL ACTIVE TO REST IN GREEN.

-RADAR DETECTION UNITS FOR DILEMMA ZONE DETECTION SHALL PLACE A CONSTANT CALL TO THE CONTROLLER WHEN VEHICLE TRAVEL TIMES TO THE STOP BAR ARE BETWEEN 2.5 AND 6.0 SECONDS. SPEED TRIGGER SHALL BE SET FOR VEHICLES TRAVELING 35 MPH AND GREATER.

-RADAR SHALL HAVE QUEUE DETECTION CONFIGURED AND A ZONE PLACED AT 100-200 FEET FROM THE STOP BAR FOR SLOW MOVING VEHICLE EXTENSIONS. SPEED TRIGGER SHALL BE SET AT 1-35 MPH.

- ALL DETECTOR DELAYS SHALL BE PLACED IN THE CONTROLLER.

PHASING DIAGRAM



RADAR DETECTION CHART

DETECTION ZONE	MOVEMENT	PULSE OR PRESENCE	ASSOCIATED PHASE	DELAY PROGRAMMED IN CONTROLLER (SEC.)	EXTENSION PROGRAMMED IN CONTROLLER (SEC.)	DETECTOR NO.	PURPOSE	DETECTION ZONE LENGTH (FT)
RDZ5A	NB LT	PRESENCE	Ø5	5.0	2.0	D2A	STOP BAR	40
RDZ2A	NB	PULSE	Ø2	0.0	2.0	D2B	DILEMMA ZONE	850
RDZ4A	EB LR	PRESENCE	Ø4	10.0	2.0	D4A	STOP BAR	40
RDZ6A	SB	PULSE	Ø6	0.0	2.0	D6A	DILEMMA ZONE	850

NOTE: ALL DETECTION ZONES SHALL BE CENTERED IN THEIR RESPECTIVE LANES UNLESS OTHERWISE SHOWN.

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TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

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T10
T14

COORDINATION TIME CHART

"INTERSECTION 1" - S.R. 149 & RECO DRIVE										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	NBLT	SB	-	-		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	40	-	20	13.4	26.6	-	-	55	-
2	-	52.6	-	17.4	13.4	39.2	-	-	64	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

"INTERSECTION 3" - I-70 WESTBOUND RAMPS & S.R. 149 (MASTER)										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	WB	-	SB	-	WB		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	37.5	-	22.5	-	37.5	-	-	0	-
2	-	43	-	27	-	43	-	-	0	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

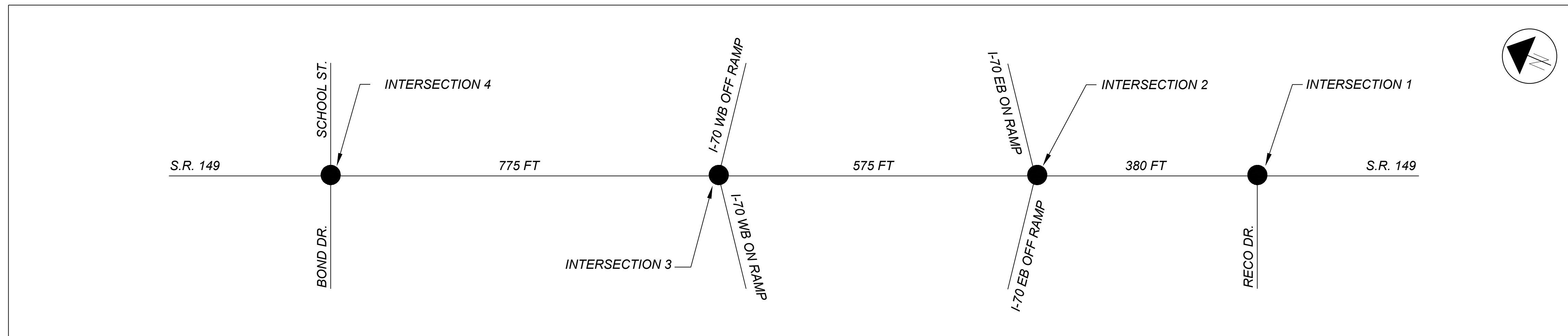
"INTERSECTION 2" - I-70 EASTBOUND RAMPS & S.R. 149										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	-	SB	-	-		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	42	-	18	-	42	-	-	52	-
2	-	47.5	-	22.5	-	47.5	-	-	62	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

"INTERSECTION 4" - S.R. 149 & BOND DRIVE/SCHOOL STREET										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	-	SB	-	WB		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	37	-	23	-	37	-	23	12	-
2	-	42	-	28	-	42	-	28	5	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

COORDINATION TIMING PLANS

DAY(S) OF WEEK	PLAN NAME	HOURS	PLAN NO. OR CYCLE/SPLIT/OFFSET	CYCLE LENGTH (SEC)
-	-	-	-	-
SAT-SUN	FREE	0000 - 2400	FREE	FREE
-	-	-	-	-
MON-FRI	FREE	0000 - 0700	FREE	FREE
MON-FRI	AM PEAK	0700 - 0930	1	60
MON-FRI	FREE	0930 - 1430	FREE	FREE
MON-FRI	PM PEAK	1430 - 1800	2	70
MON-FRI	FREE	1800 - 2400	FREE	FREE
-	-	-	-	-

COORDINATION LAYOUT



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TRAFFIC SIGNAL COORDINATION PLAN
S.R. 149

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T11
T14

SHEET NO.	REFERENCE	STATION TO STATION	625	625	625	625	625	625	625	625	625	632	632	632	632	632	632	632	632	632	633	633	633	633	633	633	809	809	809
			CONDUIT, 2", 725.04	CONDUIT, 3", 725.04	TRENCH	CONDUIT, JACKED OR DRILLED, 725.04, 4"	PULL BOX, 725.08, 18"	PULL BOX, 725.08, 24"	GROUND ROD	UNDERGROUND WARNING/MARKING TAPE	ARC FLASH CALCULATIONS AND LABEL	VEHICULAR SIGNAL HEAD, (LED), 3-SECTION, 12" LENS, 1-WAY, POLYCARBONATE	VEHICULAR SIGNAL HEAD, (LED), 5-SECTION, 12" LENS, 1-WAY, POLYCARBONATE	COVERING OF VEHICULAR SIGNAL HEAD	SIGNAL CABLE, 7 CONDUCTOR, NO. 14 AWG	SIGNAL SUPPORT FOUNDATION	POWER CABLE, 2 CONDUCTOR, NO. 6 AWG	POWER SERVICE, AS PER PLAN	CONDUIT RISER, 2" DIAMETER	SIGNAL SUPPORT, TYPE TC-81.22, DESIGN 4	SIGNAL SUPPORT, TYPE TC-81.22, DESIGN 12	CABINET, TYPE 332L, AS PER PLAN	CABINET FOUNDATION	CONTROLLER WORK PAD	UNINTERRUPTIBLE POWER SUPPLY (UPS), 1000 WATT, AS PER PLAN	CONTROLLER ITEM, MISC.; TIMING AND COORDINATION	COMMUNICATIONS, AS PER PLAN	ADVANCED RADAR DETECTION, AS PER PLAN	STOP LINE RADAR DETECTION, AS PER PLAN
			FT	FT	FT	FT	EACH	EACH	EACH	FT	EACH	EACH	EACH	FT	EACH	FT	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH	EACH
SR 149																													
T7	CONTR.	121+79.32 RT TO 121+76.48 RT		3	3			1		3						24		8				1	1	1	1				1
	PB-3	121+76.48 RT TO 121+84.88 RT	42		21																								
	SP-3	121+84.88 RT TO 121+14.66 RT		66	66				1	21		2	1	3	82.5	1	31			1								1	
	PB-2	121+14.66 RT TO 121+21.28 RT	8		8																								
	SP-2	121+21.28 RT TO 121+13.82 LT				95				1					13				1										
	PB-1	121+13.82 LT TO 120+97.59 LT	16		16																								
	SP-1	120+97.59 LT TO 124+25.34 RT																											
	POWER	124+25.34 RT TO 121+79.32 RT																											
	CONTR.	121+79.32 RT																											
TOTALS CARRIED TO GENERAL SUMMARY			66	69	114	95	2	1	4	114	1	6	1	7	543	3	39	1	2	1	2	1	1	1	1	1	2	2	1

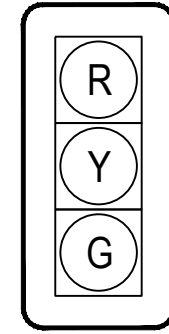
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TRAFFIC SIGNAL SUBSUMMARY
S.R. 149

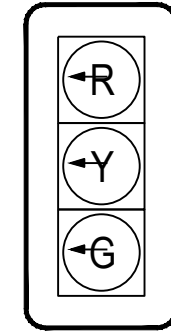
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T12
T14

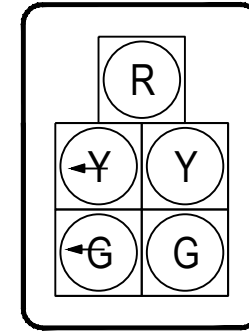
**SIGNAL HEADS, 12", LED
WITH BACK PLATES**



SIGNALS
2A, 2B, 2C,
4C, 6A, 6C

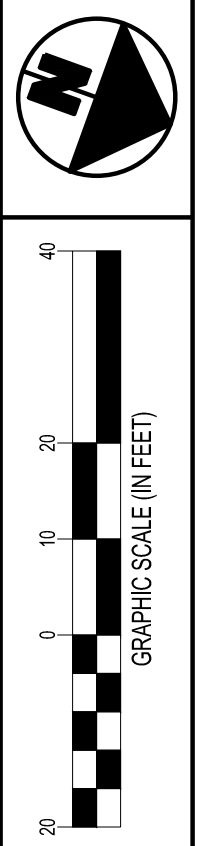


SIGNALS
4A



SIGNALS
4B, 6B

TRAFFIC SIGNAL SUBSUMMARY			
ITEM	QUANTITY	UNIT	
614	1	LS	MAINTAINING TRAFFIC
632	1	EACH	VEHICULAR SIGNAL HEAD, (LED), 5-SECTION, 12" LENS, 1-WAY, POLYCARBONATE
809	1	EACH	STOP LINE RADAR DETECTION, AS PER PLAN



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REM

EXISTING 3-SECTION SIGNAL HEAD
TO BE REPLACED WITH 5-SECTION
SIGNAL HEAD.

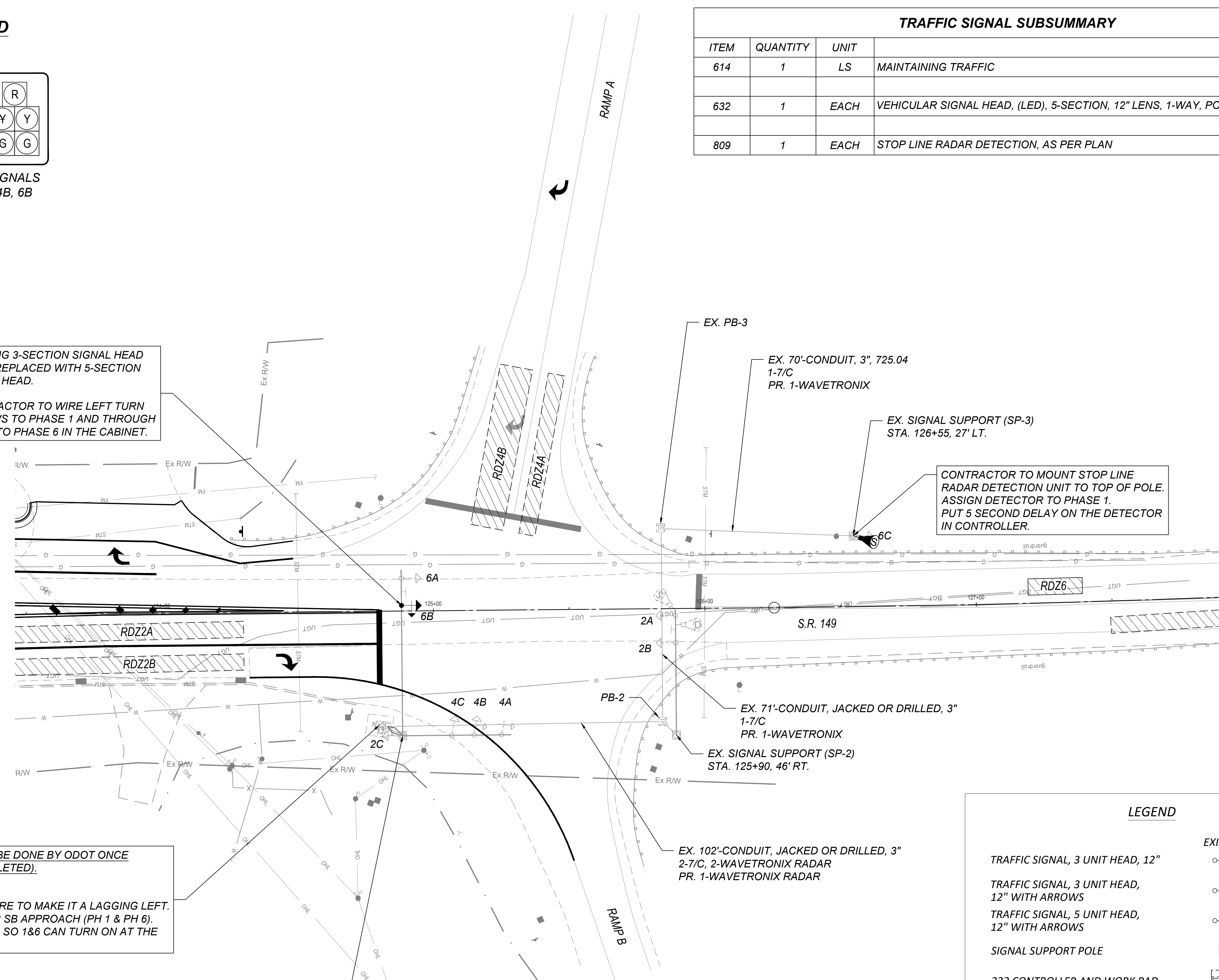
CONTRACTOR TO WIRE LEFT TURN
ARROWS TO PHASE 1 AND THROUGH
BALLS TO PHASE 6 IN THE CABINET.

CONTRACTOR TO MOUNT STOP LINE
RADAR DETECTION UNIT TO TOP OF POLE.
ASSIGN DETECTOR TO PHASE 1.
PUT 5 SECOND DELAY ON THE DETECTOR
IN CONTROLLER.

CONTROLLER WORK (TO BE DONE BY ODOT ONCE
CONSTRUCTION IS COMPLETED).

- ACTIVATE PHASE 1.
- CHANGE RING STRUCTURE TO MAKE IT A LAGGING LEFT.
- ENABLE PREEMPT 3 FOR SB APPROACH (PH 1 & PH 6).
- SOLDER MONITOR CARD SO 1&6 CAN TURN ON AT THE SAME TIME.

PULL BOX DATA			
NO.	STATION	OFFSET	SIZE
PB-1	124+82.8	42' RT.	24"
PB-2	125+85	41' RT.	18"
PB-3	125+85	30' LT.	18"



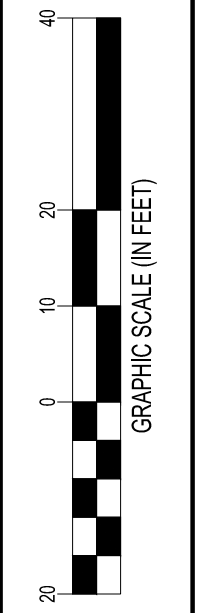
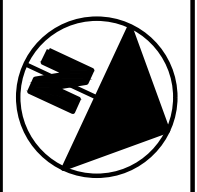
LEGEND		
	EXISTING	PROPOSED
TRAFFIC SIGNAL, 3 UNIT HEAD, 12"		
TRAFFIC SIGNAL, 3 UNIT HEAD, 12" WITH ARROWS		
TRAFFIC SIGNAL, 5 UNIT HEAD, 12" WITH ARROWS		
SIGNAL SUPPORT POLE		
332 CONTROLLER AND WORK PAD		
TRAFFIC PULL BOX		
DILEMMA ZONE RADAR DETECTION UNIT		
STOP LINE RADAR DETECTION UNIT		
DETECTION ZONE		

TRAFFIC SIGNAL PLAN
S.R. 149 & EASTBOUND I.R. 70 RAMPS

BEL-149-23.44

T13
T14

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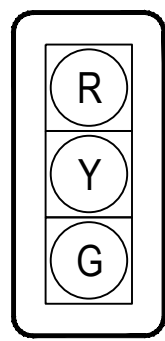
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TRAFFIC SIGNAL PLAN
S.R. 149 & WESTBOUND I.R. 70 RAMPS

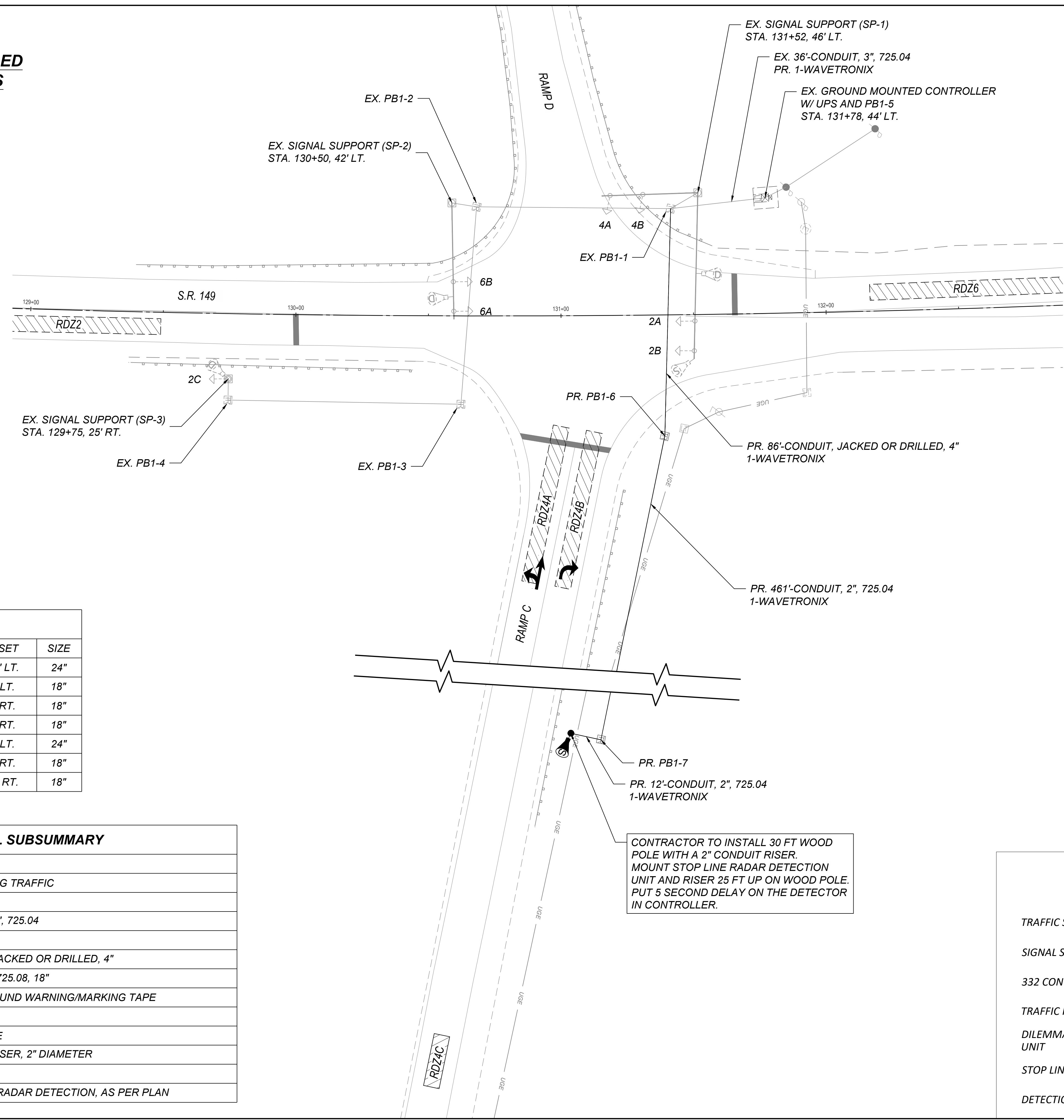
BEL-149-23.44

T14
T14

**SIGNAL HEADS, 12", LED
WITH BACK PLATES**



SIGNALS
2A, 2B, 2C,
4A, 4B, 6A, 6B



PULL BOX DATA

	NO.	STATION	OFFSET	SIZE
EX.	PB1-1	131+42	40.5' LT.	24"
EX.	PB1-2	130+68	41' LT.	18"
EX.	PB1-3	130+68	31' RT.	18"
EX.	PB1-4	129+75	30' RT.	18"
EX.	PB1-5	131+78	44' LT.	24"
PR.	PB1-6	131+39	46' RT.	18"
PR.	PB1-7	130+49	497' RT.	18"

TRAFFIC SIGNAL SUBSUMMARY

ITEM	QUANTITY	UNIT	
614	1	LS	MAINTAINING TRAFFIC
625	473	FT	CONDUIT, 2", 725.04
625	473	FT	TRENCH
625	86	FT	CONDUIT, JACKED OR DRILLED, 4"
625	2	EACH	PULL BOX, 725.08, 18"
625	473	FT	UNDERGROUND WARNING/MARKING TAPE
632	1	EACH	WOOD POLE
632	1	EACH	CONDUIT RISER, 2" DIAMETER
809	1	EACH	STOP LINE RADAR DETECTION, AS PER PLAN

CONTRACTOR TO INSTALL 30 FT WOOD POLE WITH A 2" CONDUIT RISER. MOUNT STOP LINE RADAR DETECTION UNIT AND RISER 25 FT UP ON WOOD POLE. PUT 5 SECOND DELAY ON THE DETECTOR IN CONTROLLER.

LEGEND

	EXISTING	PROPOSED
TRAFFIC SIGNAL, 3 UNIT HEAD, 12"		
SIGNAL SUPPORT POLE		
332 CONTROLLER AND WORK PAD		
TRAFFIC PULL BOX		
DILEMMA ZONE RADAR DETECTION UNIT		
STOP LINE RADAR DETECTION UNIT		
DETECTION ZONE		

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ROADWAY IMPROVEMENTS LOVE'S TRAVEL STOPS BEL-149-23.44

UNION TOWNSHIP BELMONT COUNTY, OH

INDEX OF SHEETS

TITLE SHEET	R1
SCHEMATIC PLAN	R2
GENERAL NOTES	R3
TYPICAL SECTIONS	R4-R6
MAINTENANCE OF TRAFFIC	R7-R8
SITE PLAN	R13
PLAN AND PROFILE	R14-R17
CROSS SECTIONS	R18-R26
INTERSECTION DETAILS	R27-R29
PAVEMENT MARKING AND SIGNAGE PLAN	R30-R31
CONSTRUCTION DETAILS	R32
TRAFFIC SIGNAL PLAN	T1-T13

PROJECT DESCRIPTION

CONSTRUCTION TO WIDEN EAST AND WEST SIDE OF S.R. 149 TO PROVIDE 175' NB LEFT TURN LANE INTO LOVE'S DRIVEWAY, 175' SB RIGHT-TURN LANE INTO LOVE'S DRIVEWAY, 164' TWLTL BETWEEN SOUTH PILOT DRIVEWAY & NORTH PILOT DRIVEWAY, 125' NB LEFT TURN LANE INTO RECO DRIVE, 200' SB RIGHT TURN LANE INTO RECO DRIVE AND ASSOCIATED LANE RECONFIGURATION TO ACCOMMODATE AFOREMENTIONED IMPROVEMENTS. CONSTRUCTION TO WIDEN RECO DRIVE AND INSTALLATION OF TRAFFIC SIGNAL AT S.R. 149 & RECO DRIVE INTERSECTION. DITCH AND SLOPE GRADING AND INSTALLATION OF NECESSARY TRAFFIC CONTROL DEVICES. STORMWATER STRUCTURE AND CONDUIT MODIFICATIONS BETWEEN SOUTH MARATHON DRIVEWAY AND NORTH MARATHON DRIVEWAY.

EARTH DISTURBED AREAS AND TOTAL AREA OF PROJECT

TOTAL AREA OF PROJECT: 2.07 ACRES
PROJECT EARTH DISTURBING ACTIVITIES AREA: 1.77 ACRES
CONTRACTOR EARTH DISTURBING ACTIVITIES AREA: 0.30 ACRES
NOI EARTH DISTURBING ACTIVITIES AREA: 2.07 ACRES

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY AND THAT PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE AS SET FORTH ON THE PLANS AND ESTIMATES.

APPROVED _____

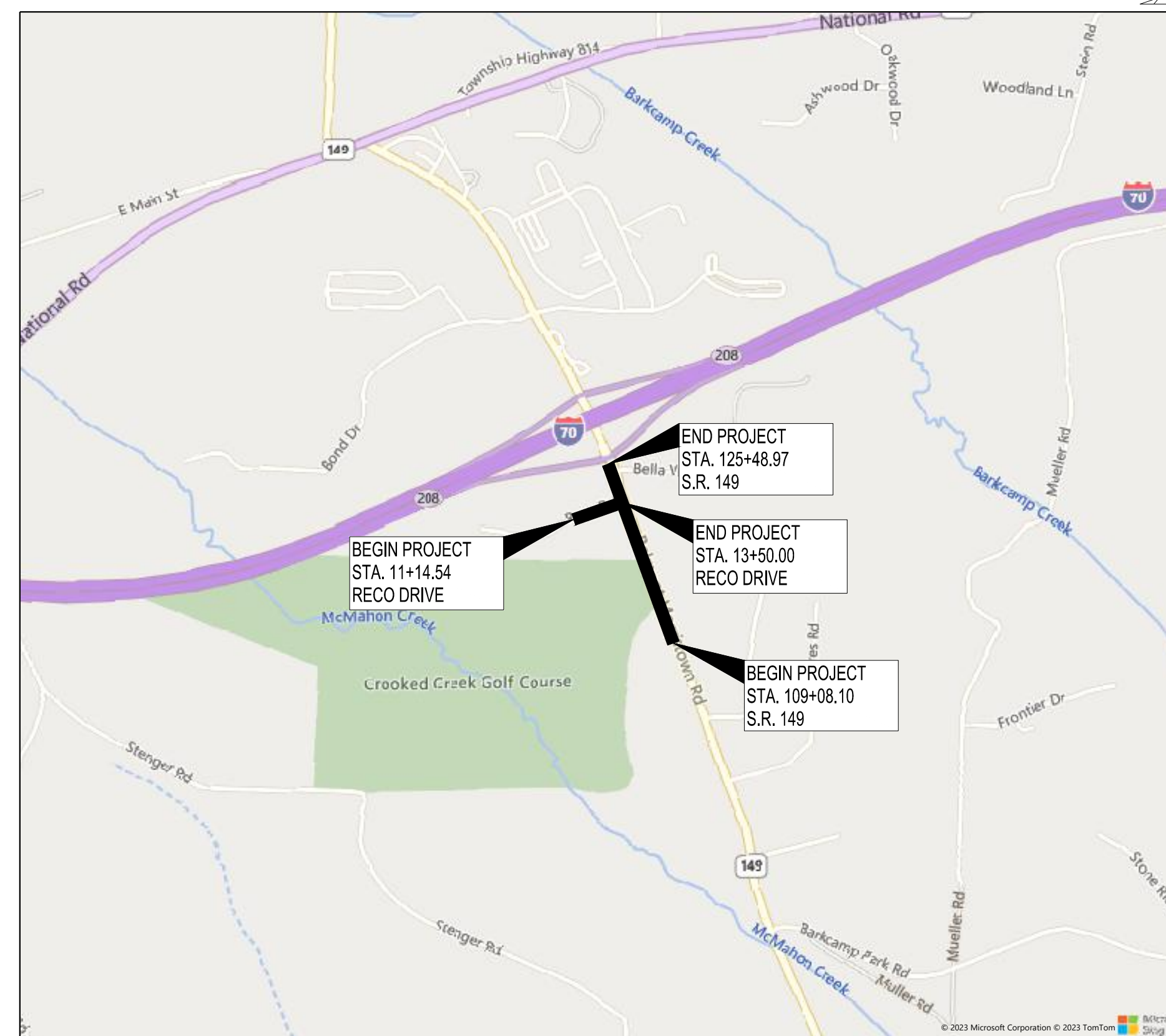
DATE _____ DISTRICT DEPUTY DIRECTOR

APPROVED _____

DATE _____ DIRECTOR, DEPARTMENT OF TRANSPORTATION

RAILROAD INVOLVEMENT
NONE

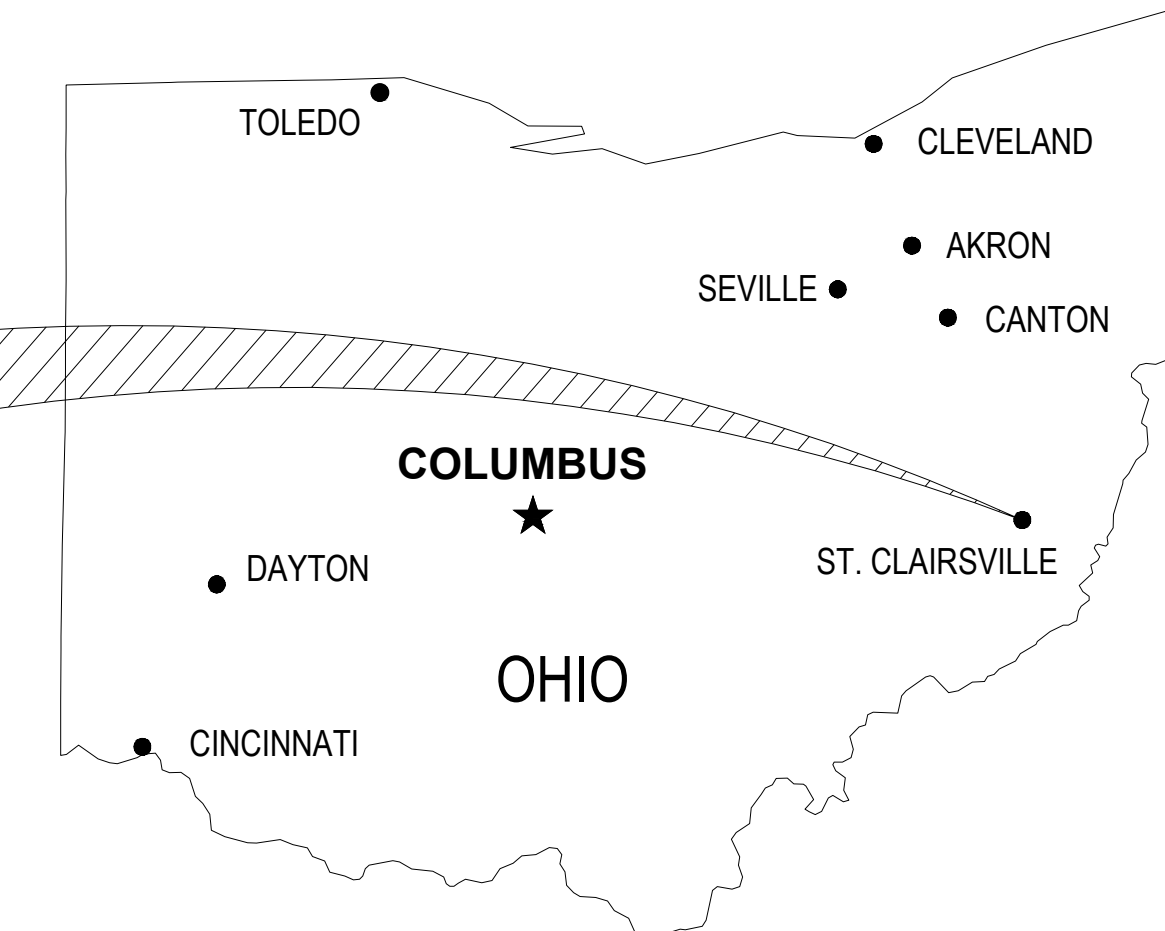
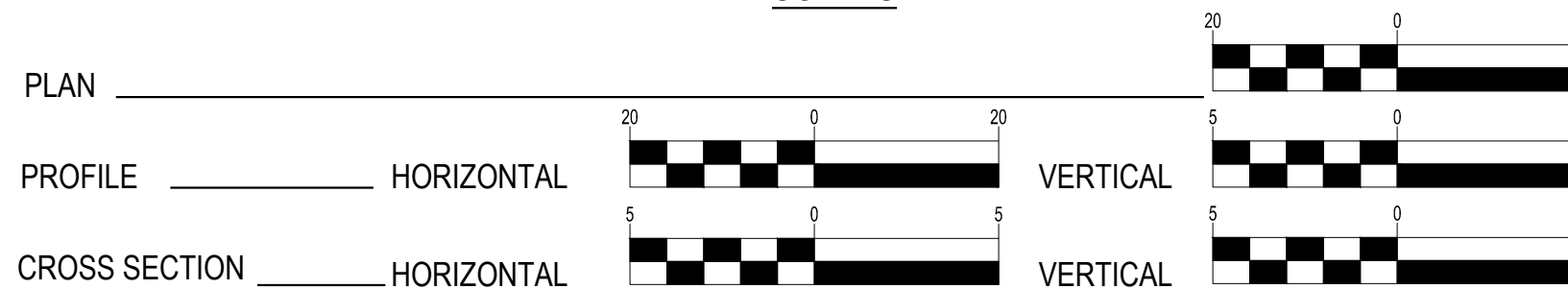
FEDERAL PROJECT NUMBER
NON-FEDERAL



LOCATION MAP
NOT TO SCALE

PROJECT LOCATED ALONG S.R. 149 - BELMONT MORRISTOWN ROAD, 1.29 MILES SOUTHEAST OF MORRISTOWN, OHIO AND SOUTH OF THE THE S.R. 149 AND I-70 INTERCHANGE. APPROXIMATELY 8.00 MILES WEST OF CLAIRSVILLE, OHIO

SCALES



STATE LOCATION MAP
NOT TO SCALE

SITE LOCATION

LAT = N 40° 03' 27"
LONG = W 81° 03' 09"

DESIGN DESIGNATION - STATE ROUTE 149 (BELMONT MORRISTOWN ROAD):

AADT (2024)	10,000
DESIGN YEAR AADT (2044)	14,500
DESIGN HOURLY VOLUME (2044)	1,300
DIRECTIONAL DISTRIBUTION	55%
TRUCKS (24 HOUR B&C)	21%
DESIGN SPEED	50 MPH
LEGAL SPEED	45 MPH
DESIGN FUNCTIONAL CLASSIFICATION	5 - RURAL MAJOR COLLECTOR
NHS PROJECT	NO

2023 SPECIFICATIONS

THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO DEPARTMENT OF TRANSPORTATION INCLUDING CHANGES AND SUPPLEMENTAL SPECIFICATIONS LISTED IN PLANS AND CHANGES LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

PLAN PREPARED BY:



ENGINEER'S STAMP	STANDARD CONSTRUCTION DRAWINGS								SUPPLEMENTAL SPECIFICATIONS		REVISIONS				
	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DESCRIPTION			
	ODOT HW-2.2	7/20/2018	ODOT DM-1.1	7/17/2020	ODOT MT-95.31	7/19/2019	ODOT TC-41.41	7/19/2019	ODOT TC-81.22	7/21/2023	SS 800	1/19/2024	PD	11/8/2023	ODOT COMMENTS #1
	ODOT CB-3A	7/16/2021	ODOT DM-4.3	1/15/2016	ODOT MT-97.10	4/19/2019	ODOT TC-42.20	10/18/2013	ODOT TC-83.10	1/17/2020	SS 832	7/21/2023	PD	1/2/2024	ODOT COMMENTS #2
	ODOT BP-3.1	1/21/2022	ODOT DM-4.4	1/15/2016	ODOT MT-99.20	7/19/2019	ODOT TC-52.10	10/18/2013	ODOT TC-83.20	1/19/2024	SS 863	7/21/2023	PD	1/3/2024	REV 1
	ODOT BP-4.1	7/19/2013	ODOT MGS-1.1	7/16/2021	ODOT MT-101.90	7/17/2020	ODOT TC-61.30	7/19/2019	ODOT TC-85.20	4/1/2023	SS 809	1/19/2024	PD	2/9/2024	ADDENDUM #3
	ODOT BP-5.1	7/15/2022	ODOT MGS-2.1	1/19/2018	ODOT MT-120.00	1/19/2024							PD	2/23/2024	ODOT COMMENTS #3
			ODOT MGS-4.2	7/19/2013	ODOT TC-21.21	1/20/2023	ODOT TC-65.10	1/17/2014					PD	2/23/2024	ADDENDUM #4
					ODOT TC-41.20	10/18/2013	ODOT TC-65.11	7/15/2022					PD	4/29/2024	ODOT COMMENTS #4
					ODOT TC-41.30	4/21/2023	ODOT TC-71.10	4/21/2023					PD	6/24/2024	CCD07
													PD	8/1/2024	CCD09
													PD	8/12/2024	CCD10
													PD	9/20/2024	CCD11

UNDERGROUND UTILITIES
Contact Two Working Days Before You Dig
 OHIO811, 8-1-1, or 1-800-382-2764 (Non-members must be called directly)

DATE
9-20-2024

ISSUE
AGENCY REVIEW

CALCULATED
P.I.D.
CHECKED
TDH

TITLE SHEET
S.R. 149

BEL-149-23.44

R1
R32

BENCHMARKS	
BM #A	*"X" CHISELED IN THE SOUTH SIDE OF RIM OF SANITARY MANHOLE ON NORTH SIDE OF RECO DRIVE, EAST OF ADDRESS 41245 RECO DRIVE ELEV=1239.44'
BM #B	EAST BOLT ON HYDRANT ON SOUTH SIDE OF RECO DRIVE, ACROSS FROM ADDRESS 41371 RECO DRIVE ELEV=1207.05'
BM #C	SOUTHWEST BOLT ON HYDRANT ON EAST SIDE OF BELMONT MORRISTOWN ROAD AT ADDRESS 66377 BELMONT MORRISTOWN ROAD ELEV=1193.84'

BENCHMARK NOTES:
 BM "A" AND BM "B" LOCATED OUTSIDE OF PROJECT VIEWPORT LIMITS. REFER TO ORIGINAL SURVEY CONDUCTED BY CESO, INC. DATED APRIL 12, 2023 FOR MARKED LOCATION OF BENCHMARKS.

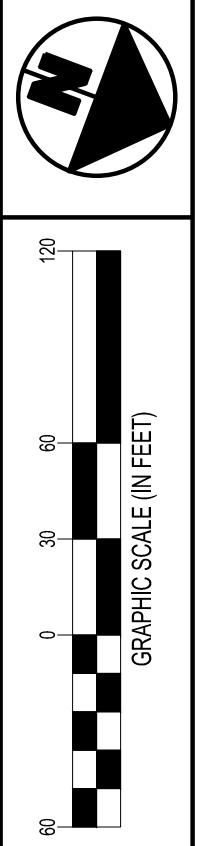
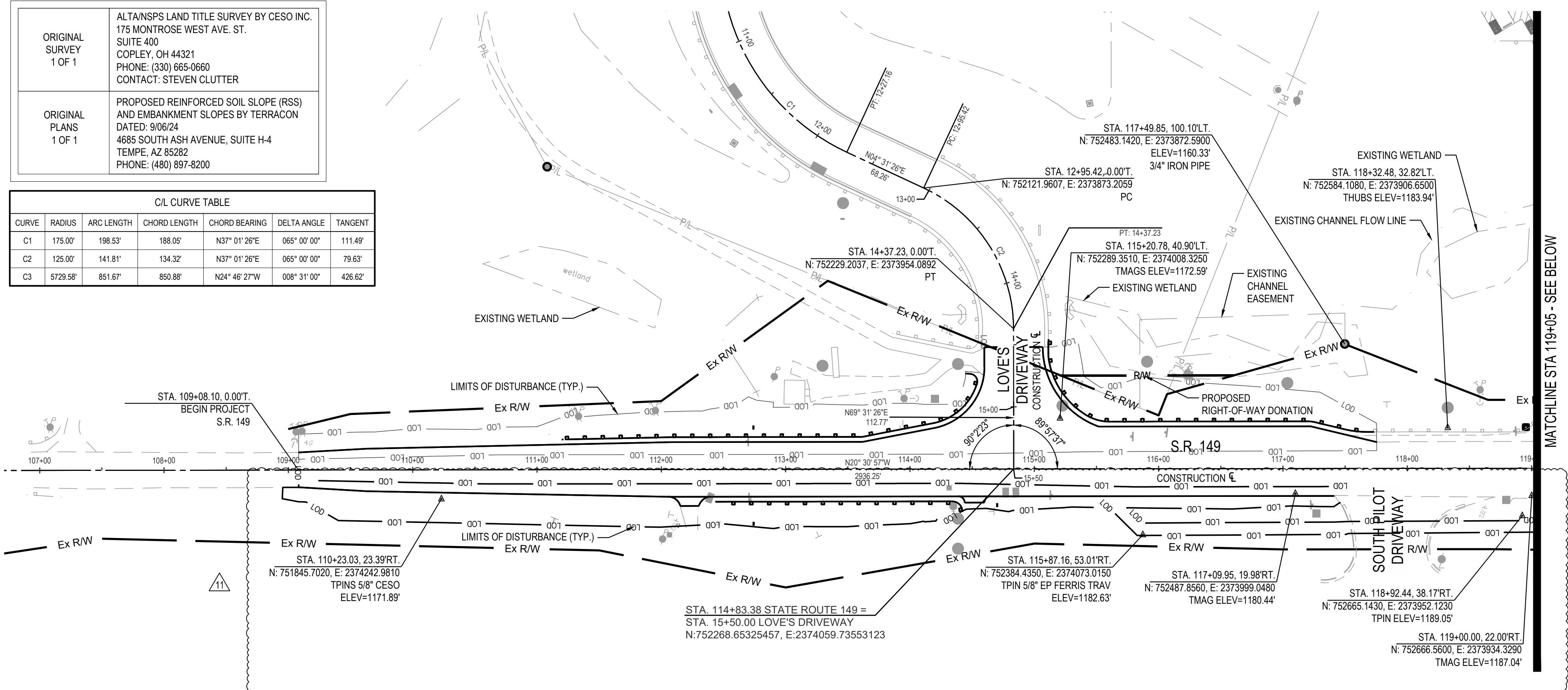
DATUM NOTES:
HORIZONTAL DATUM:
 OHIO STATE PLANE COORDINATE SYSTEM, OHIO SOUTH ZONE, NAD83 (2011) AS DETERMINED BY OPUS POST PROCESSED STATIC SESSION.
VERTICAL DATUM:
 NAVD 88 AS DETERMINED BY OPUS POST PROCESSED STATIC SESSIONS USING GEOID 18 (CONUS)
UTILITY DISCLAIMER:
 THE UTILITIES SHOWN HEREON HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION OUPS #A218001060 AND/OR EXISTING DRAWINGS. THE SURVEYOR MAKES NO GUARANTEE THAT THE UTILITIES LOCATED HERE ON COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UTILITIES LOCATED ARE IN THE EXACT LOCATION INDICATED ALTHOUGH HE DOES CERTIFY THAT THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE.
 DATE OF FIELD SURVEY: AUGUST 9, 2022
 DATE OF SURVEY MAP: APRIL 12, 2023

LANDSCAPE NOTE:
 THERE ARE NO EXISTING LANDSCAPED AREAS WITHIN THE WORK LIMITS.

REFERENCE DRAWINGS:

ORIGINAL SURVEY 1 OF 1	ALTA/SPS LAND TITLE SURVEY BY CESO INC. 175 MONTROSE WEST AVE. ST. SUITE 400 COPLEYP, OH 44321 PHONE: (330) 665-0660 CONTACT: STEVEN CLUTTER
ORIGINAL PLANS 1 OF 1	PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES BY TERRACON DATED: 9/06/24 4685 SOUTH ASH AVENUE, SUITE H-4 TEMPE, AZ 85282 PHONE: (480) 897-8200

C/L CURVE TABLE						
CURVE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE	TANGENT
C1	175.00'	198.53'	188.05'	N37° 01' 26"E	065° 00' 00"	111.49'
C2	125.00'	141.81'	134.32'	N37° 01' 26"E	065° 00' 00"	79.63'
C3	5729.58'	851.67'	850.88'	N24° 46' 27"W	008° 31' 00"	426.62'



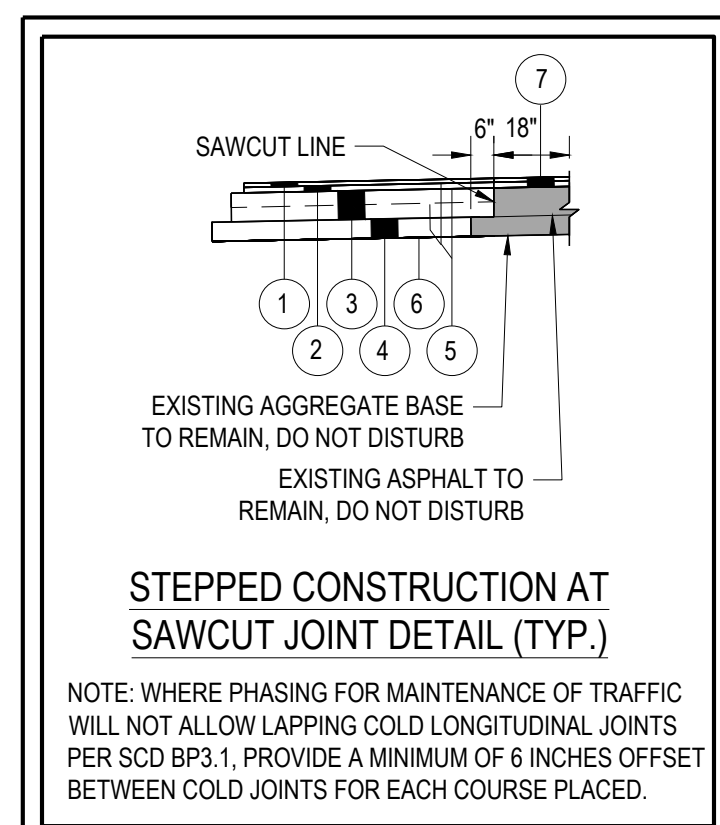
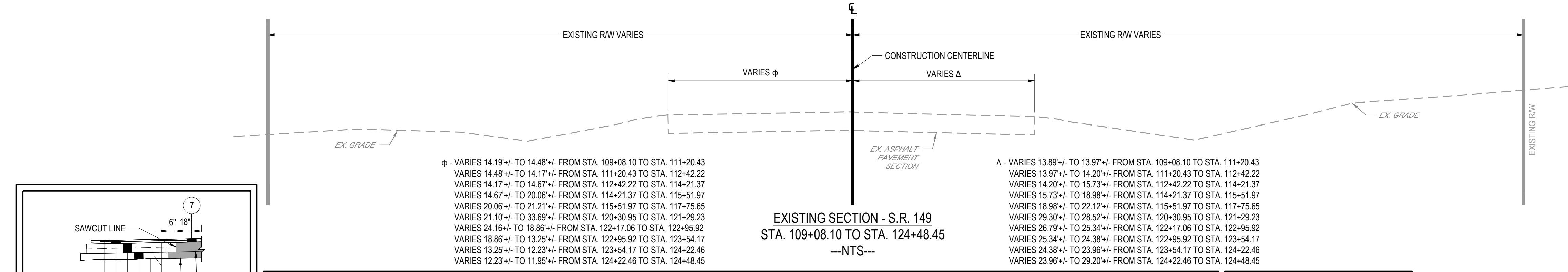
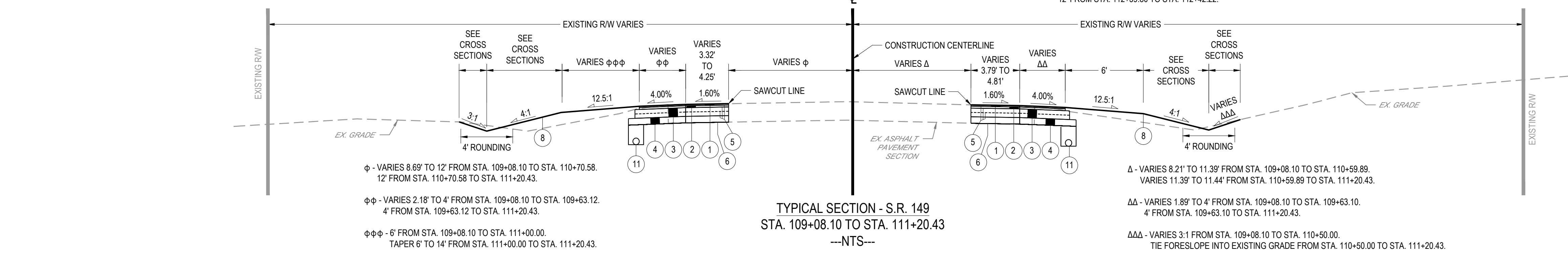
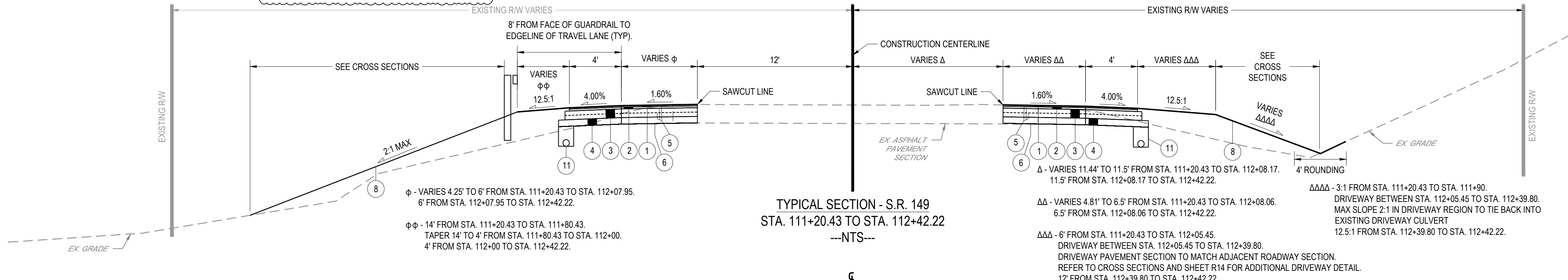
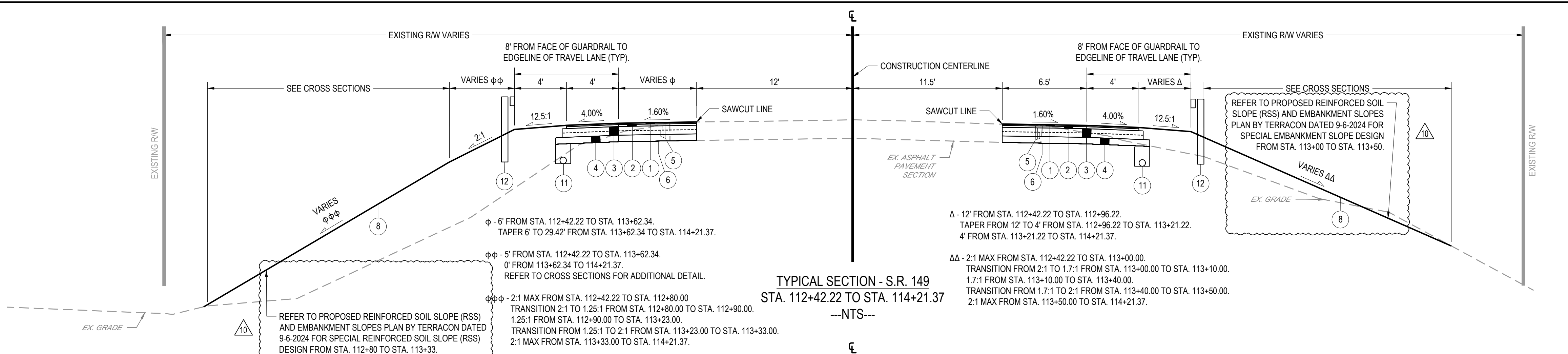
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SCHEMATIC PLAN
 S.R. 149

BEL-149-23.44

R2
 R32

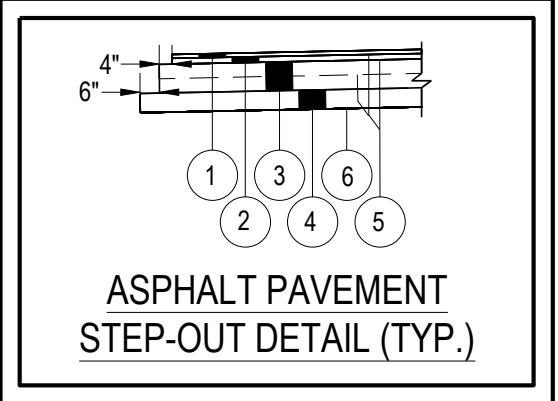
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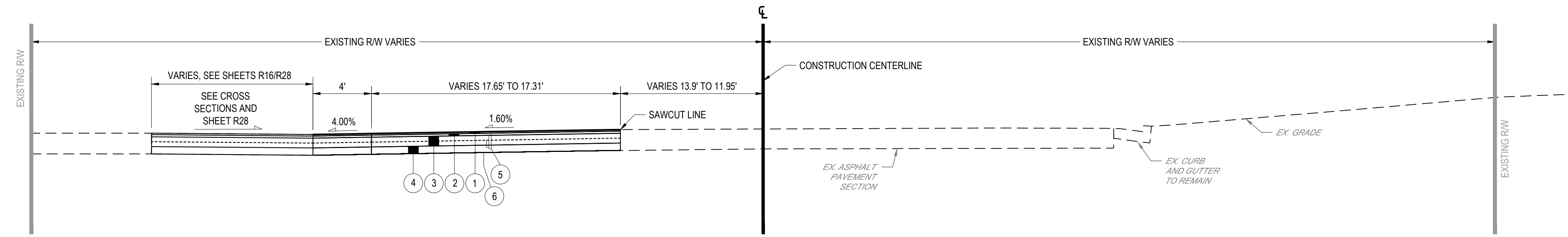
LEGEND

1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)	8	ODOT ITEM 659 - SEEDING AND MULCHING
2	ODOT ITEM 441 - 1-3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)	9	ODOT ITEM 609 - ODOT TYPE 2 COMBINATION CURB AND GUTTER PER BP-5.1
3	ODOT ITEM 301 - 8" ASPHALT CONCRETE BASE, PG64-22 (TWO 4" LIFTS, ITEM 407 BETWEEN LIFT)	10	ODOT ITEM 411 - 6" STABILIZED CRUSHED AGGREGATE WITH ODOT ITEM 408 - PRIME COAT, AS PER PLAN - SEE ITEM 408 NOTE
4	ODOT ITEM 304 - 6" AGGREGATE BASE	11	ODOT ITEM 605 - 6" BASE PIPE UNDERDRAIN WITH GEOTEXTILE FABRIC, TYPE A, 18" DEPTH TO PIPE BOTTOM FROM SUBGRADE (TYP). DEPTH VARIES AT SELECTED LOCATIONS. SEE CROSS SECTIONS FOR UNDERDRAIN ELEVATIONS/OFFSET.
5	ODOT ITEM 407 - TACK COAT	12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
6	ODOT ITEM 204 - SUBGRADE COMPACTION		---
7	ODOT ITEM 254 - 3" PAVEMENT PLANING (MILL OF EX. SURFACE AND INTERMEDIATE COURSE)		---

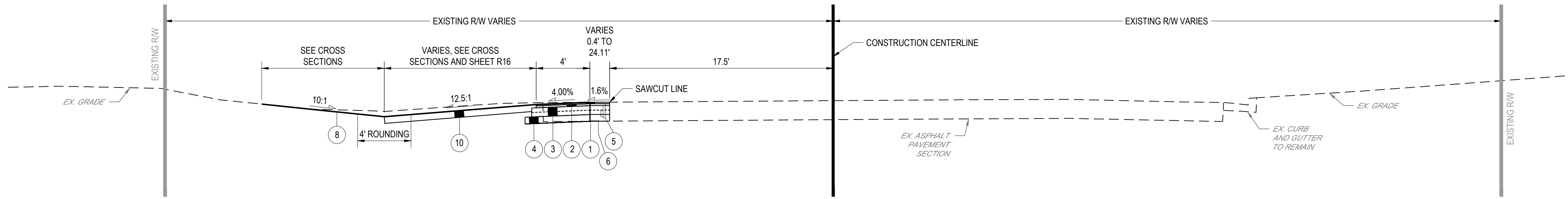
--- EXISTING GRADE/PAVEMENT SECTION



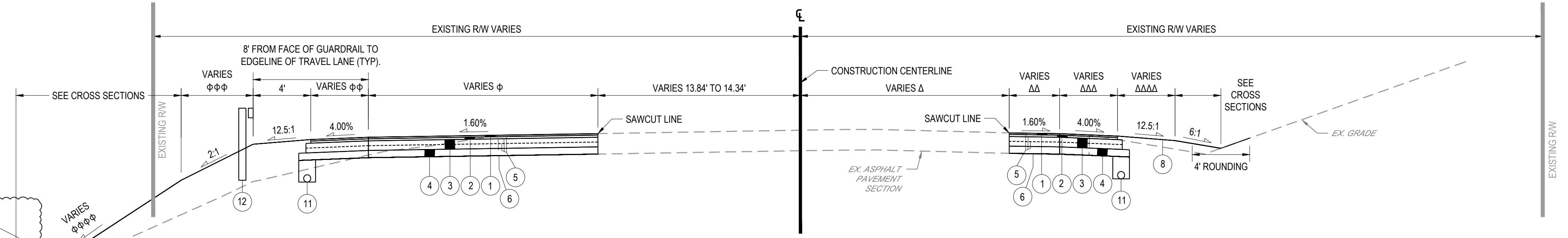
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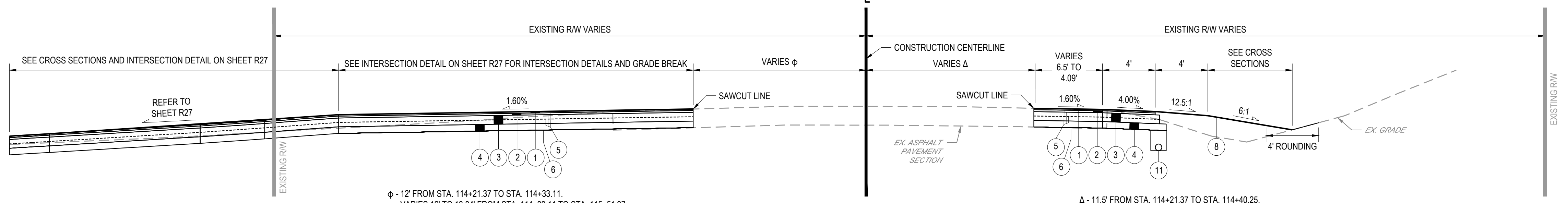
TYPICAL SECTION - S.R. 149
STA. 122+17.06 TO STA. 122+95.92
---NTS---



TYPICAL SECTION - S.R. 149
STA. 120+30.95 TO STA. 121+29.23
---NTS---

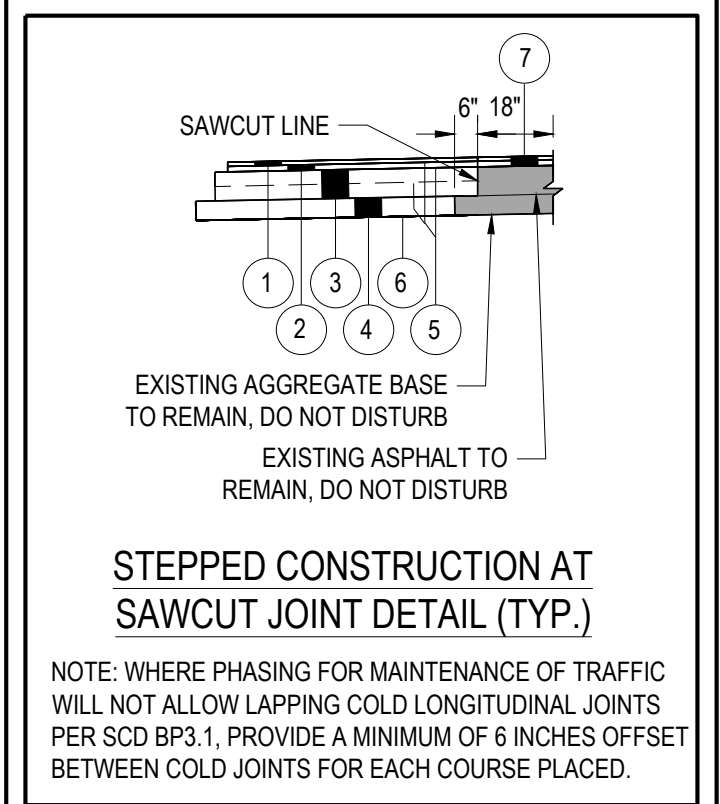


TYPICAL SECTION - S.R. 149
STA. 115+51.97 TO STA. 117+75.65
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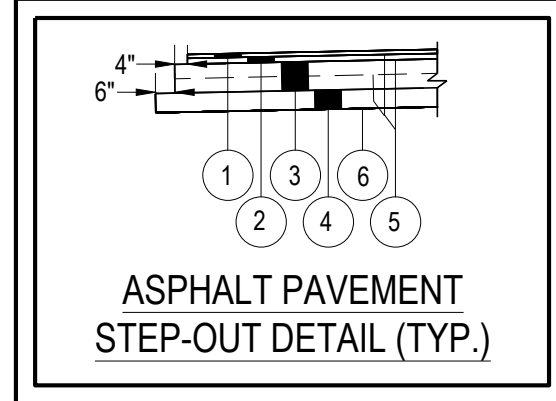


TYPICAL SECTION - S.R. 149
STA. 114+21.37 TO STA. 115+51.97
---NTS---

REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 9-6-2024 FOR SPECIAL EMBANKMENT SLOPE DESIGN FROM STA. 115+73 TO STA. 117+35.



LEGEND	
1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)
2	ODOT ITEM 441 - 1-3/4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 2, (448)
3	ODOT ITEM 301 - 8" ASPHALT CONCRETE BASE, PG64-22 (TWO 4" LIFTS, ITEM 407 BETWEEN LIFT)
4	ODOT ITEM 304 - 6" AGGREGATE BASE
5	ODOT ITEM 407 - TACK COAT
6	ODOT ITEM 204 - SUBGRADE COMPACTION
7	ODOT ITEM 254 - 3" PAVEMENT PLANING (MILL OF EX. SURFACE AND INTERMEDIATE COURSE)
8	ODOT ITEM 659 - SEEDING AND MULCHING
9	ODOT ITEM 609 - ODOT TYPE 2 COMBINATION CURB AND GUTTER PER BP-5.1
10	ODOT ITEM 411 - 6" STABILIZED CRUSHED AGGREGATE WITH ODOT ITEM 408 - PRIME COAT, AS PER PLAN - SEE ITEM 408 NOTE
11	ODOT ITEM 605 - 6" BASE PIPE UNDERDRAIN WITH GEOTEXTILE FABRIC, TYPE A, 18" DEPTH TO PIPE BOTTOM FROM SUBGRADE (TYP). DEPTH VARIES AT SELECTED LOCATIONS. SEE CROSS SECTIONS FOR UNDERDRAIN ELEVATIONS/OFFSET.
12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
---	EXISTING GRADE/PAVEMENT SECTION



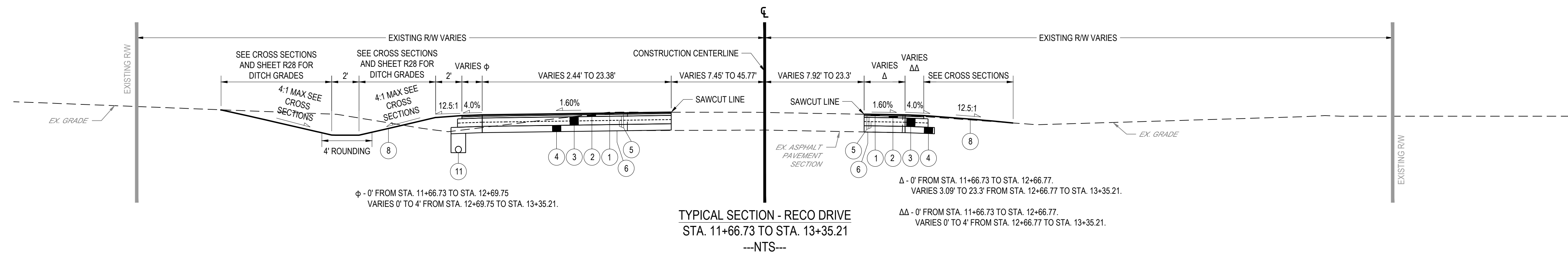
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TYPICAL SECTIONS
S.R. 149

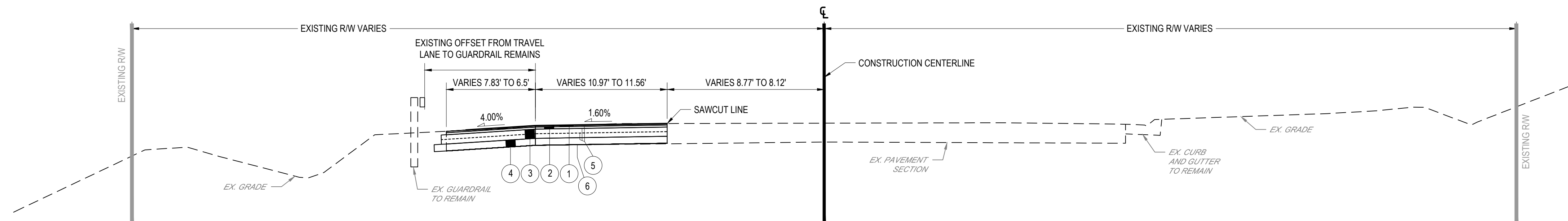
BEL-149-23.44

R5
R32

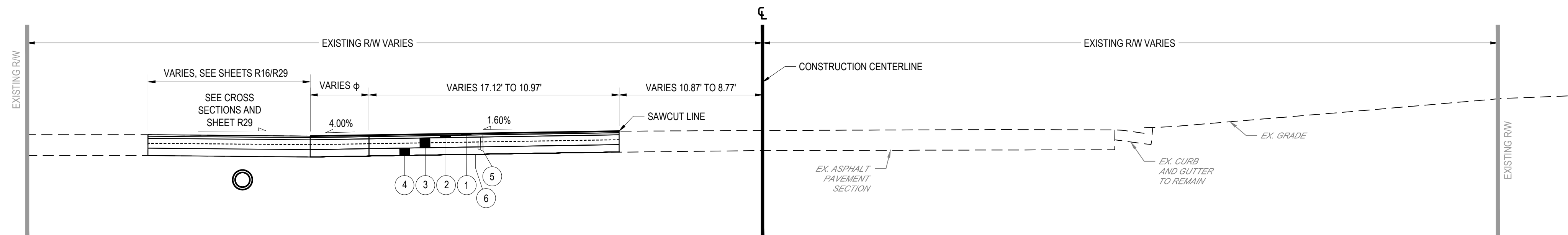
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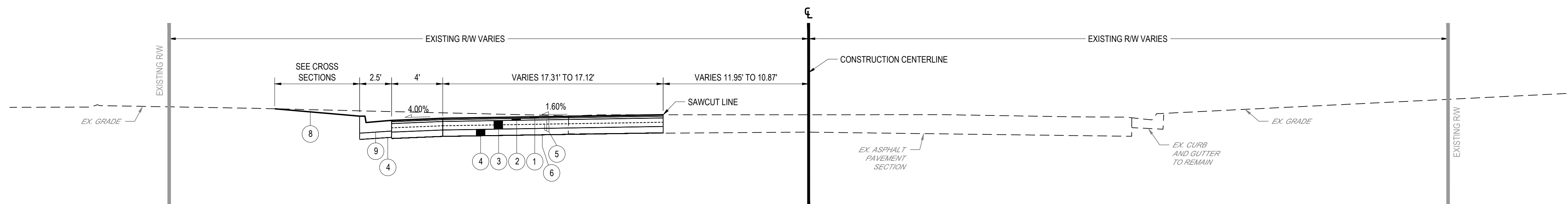
TYPICAL SECTION - RECO DRIVE
STA. 11+66.73 TO STA. 13+35.21
---NTS---



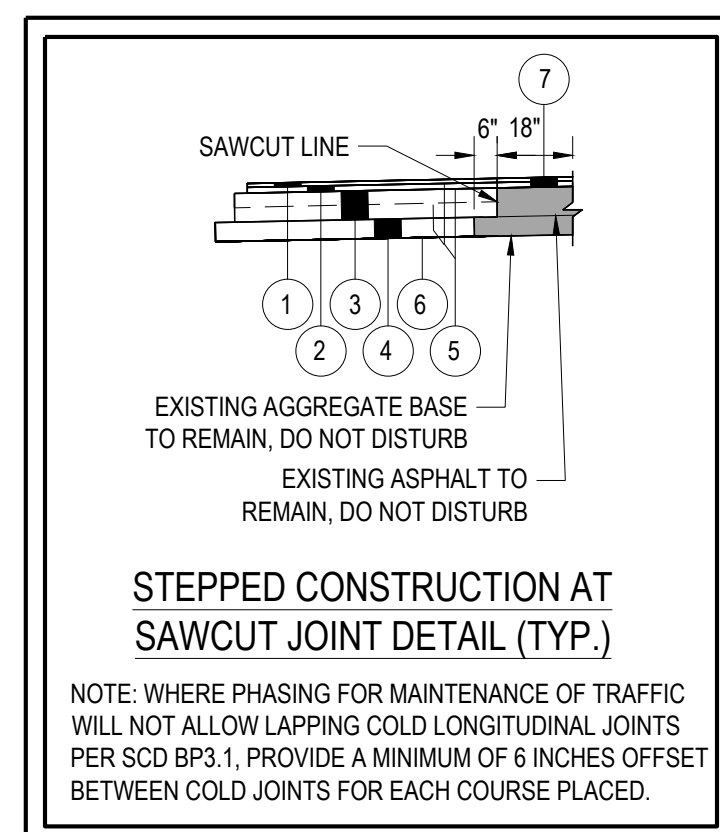
TYPICAL SECTION - S.R. 149
STA. 124+22.46 TO STA. 124+48.45
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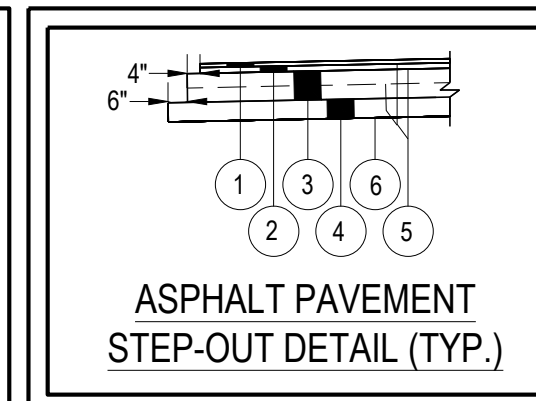
TYPICAL SECTION - S.R. 149
STA. 123+54.17 TO STA. 124+22.46
---NTS---



TYPICAL SECTION - S.R. 149
STA. 122+95.92 TO STA. 123+54.17
---NTS---



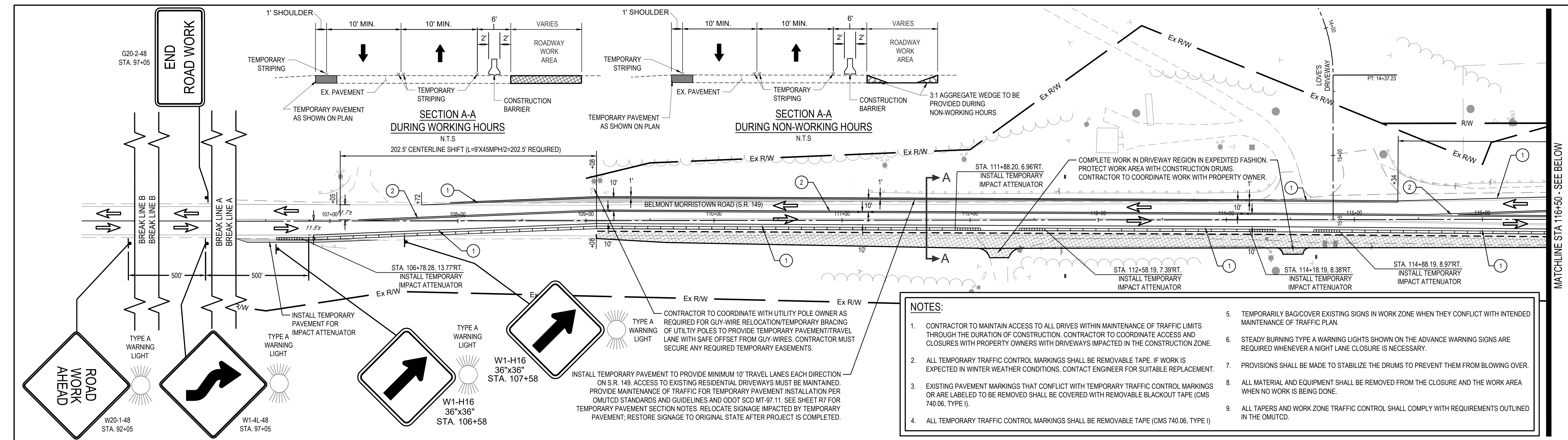
LEGEND	
1	ODOT ITEM 441 - 1-1/4" ASPHALT CONCRETE SURFACE COURSE, TYPE 1 (448), AS PER PLAN (PG70-22M)
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12	ITEM 606 - GUARDRAIL, TYPE MGS WITH LONG POSTS
---	EXISTING GRADE/PAVEMENT SECTION



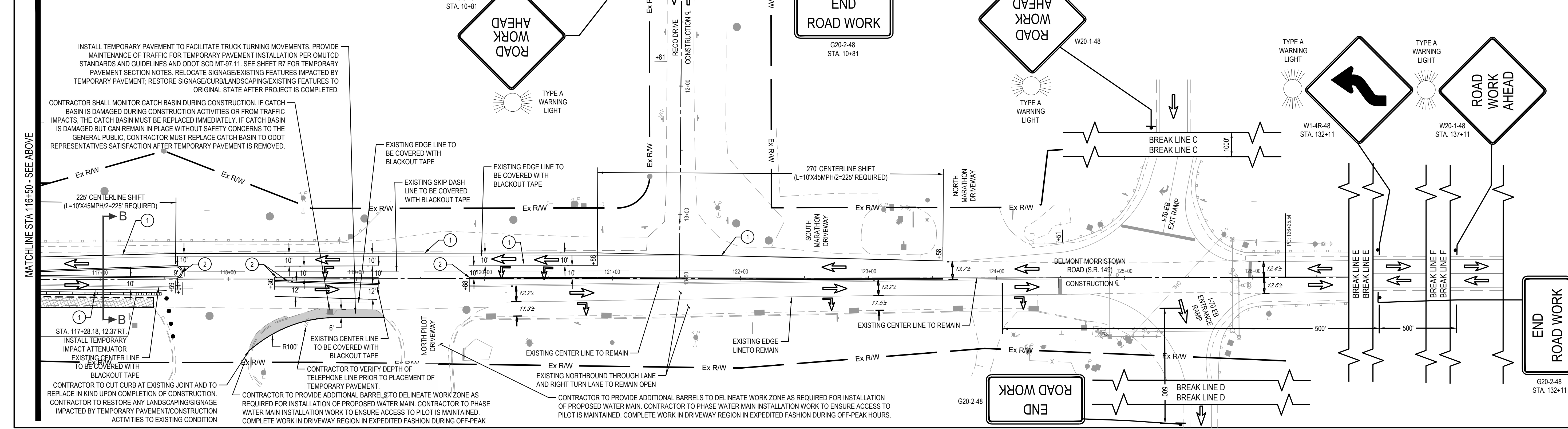
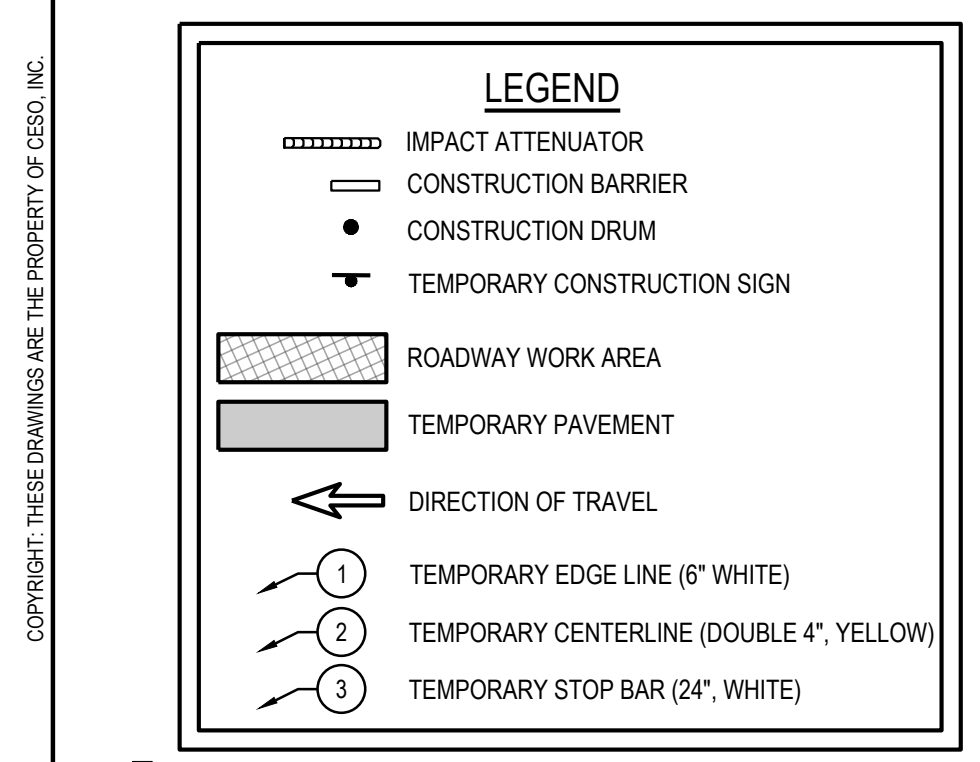
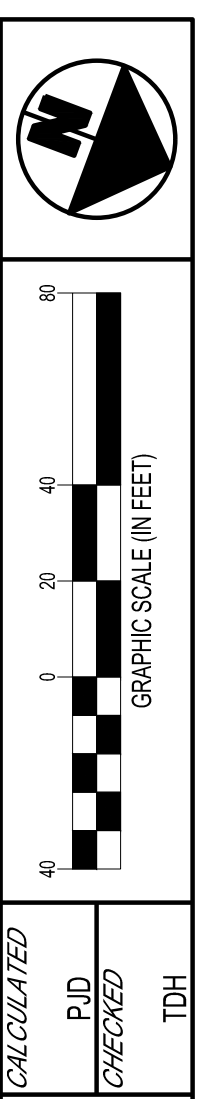
TYPICAL SECTIONS
S.R. 149

BEL-149-23.44

R6
R32



- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
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 - ALL TAPERS AND WORK ZONE TRAFFIC CONTROL SHALL COMPLY WITH REQUIREMENTS OUTLINED IN THE OMUTCD.



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MAINTENANCE OF TRAFFIC - PHASE 1
S.R. 149

BEL-149-23.44

R8
R32



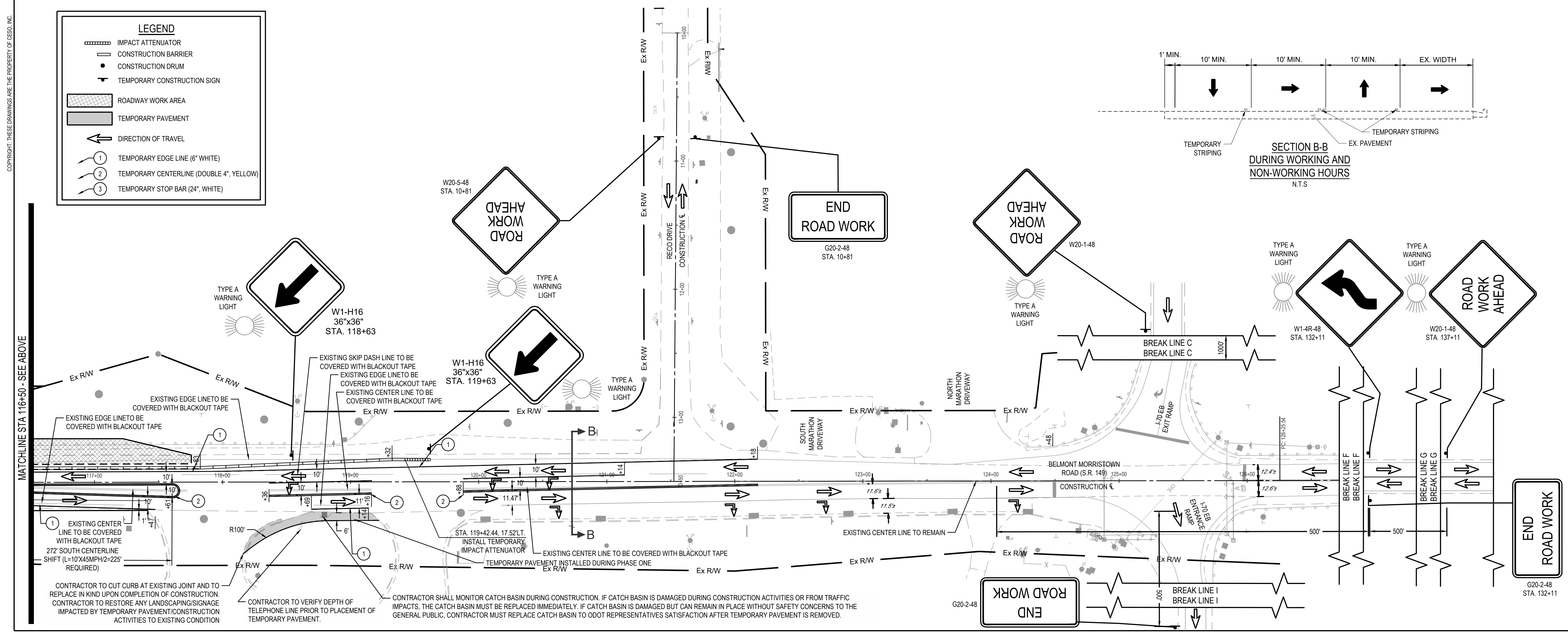
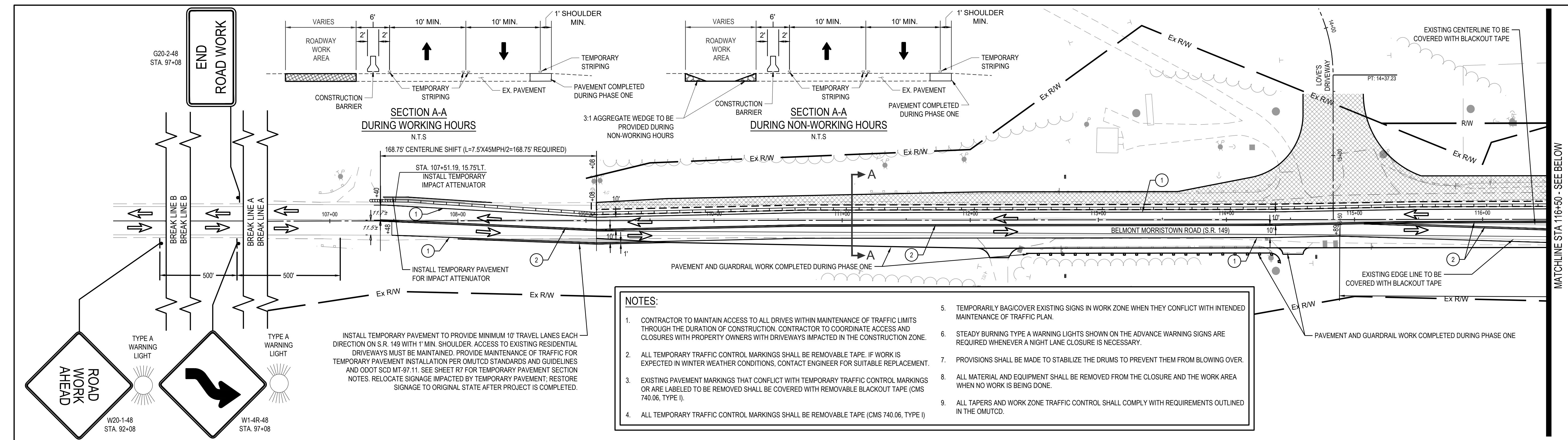
GRAPHIC SCALE (IN FEET)

CALCULATED
P.I.D.
CHECKED
TDH

MAINTENANCE OF TRAFFIC - PHASE 2
S.R. 149

BEL-149-23.44

R9
R32



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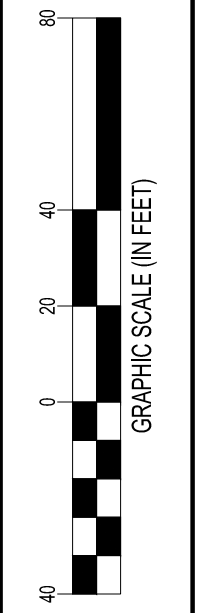
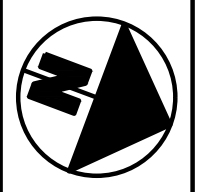
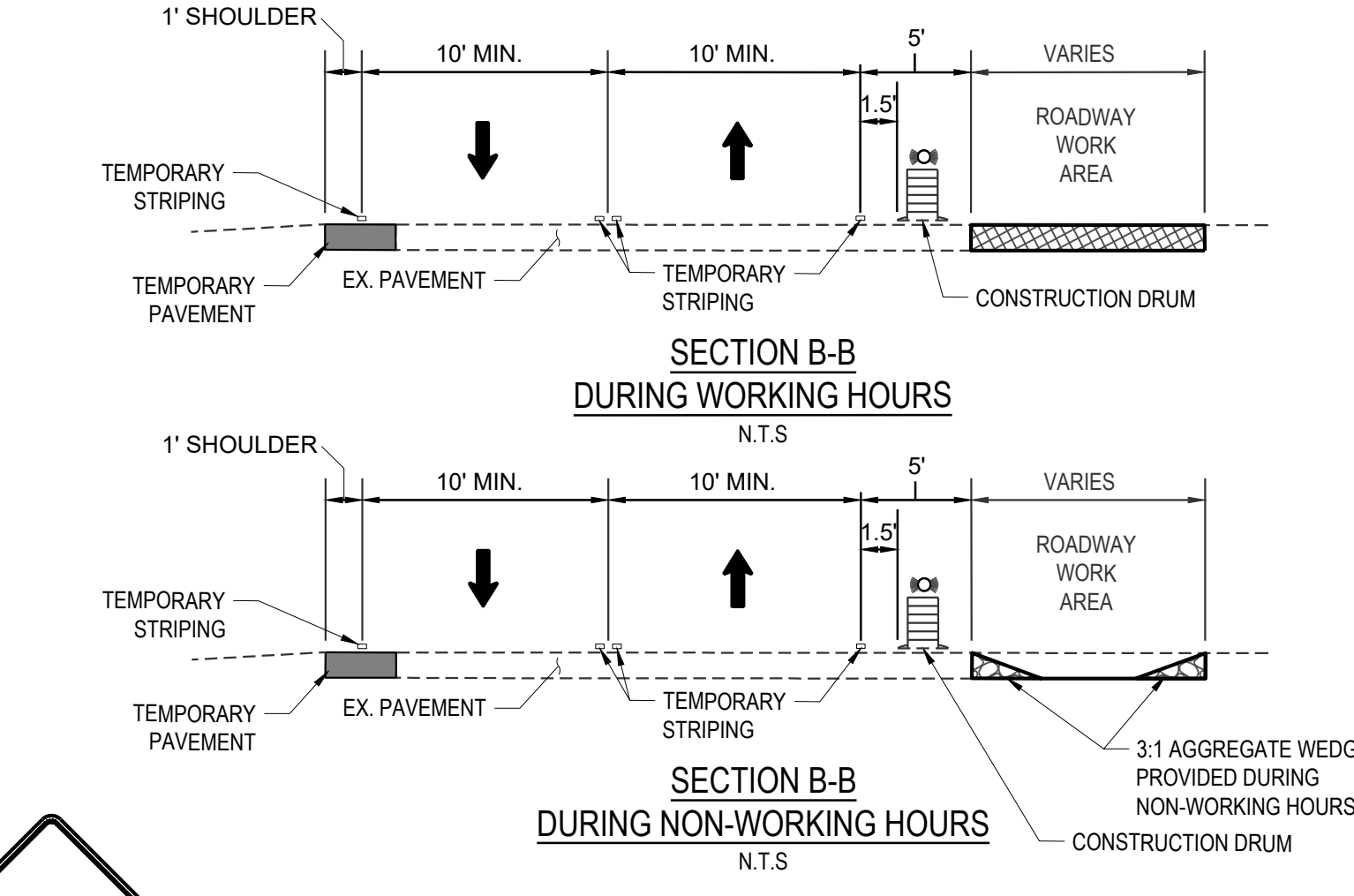
MATCHLINE STA 116+50 - SEE BELOW

MATCHLINE STA 116+50 - SEE ABOVE

- NOTES:**
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LEGEND

- CONSTRUCTION DRUM
- TEMPORARY CONSTRUCTION SIGN
- ROADWAY WORK AREA
- TEMPORARY PAVEMENT
- DIRECTION OF TRAVEL
- TEMPORARY EDGE LINE (6" WHITE)
- TEMPORARY CENTERLINE (DOUBLE 4", YELLOW)
- TEMPORARY STOP BAR (24", WHITE)

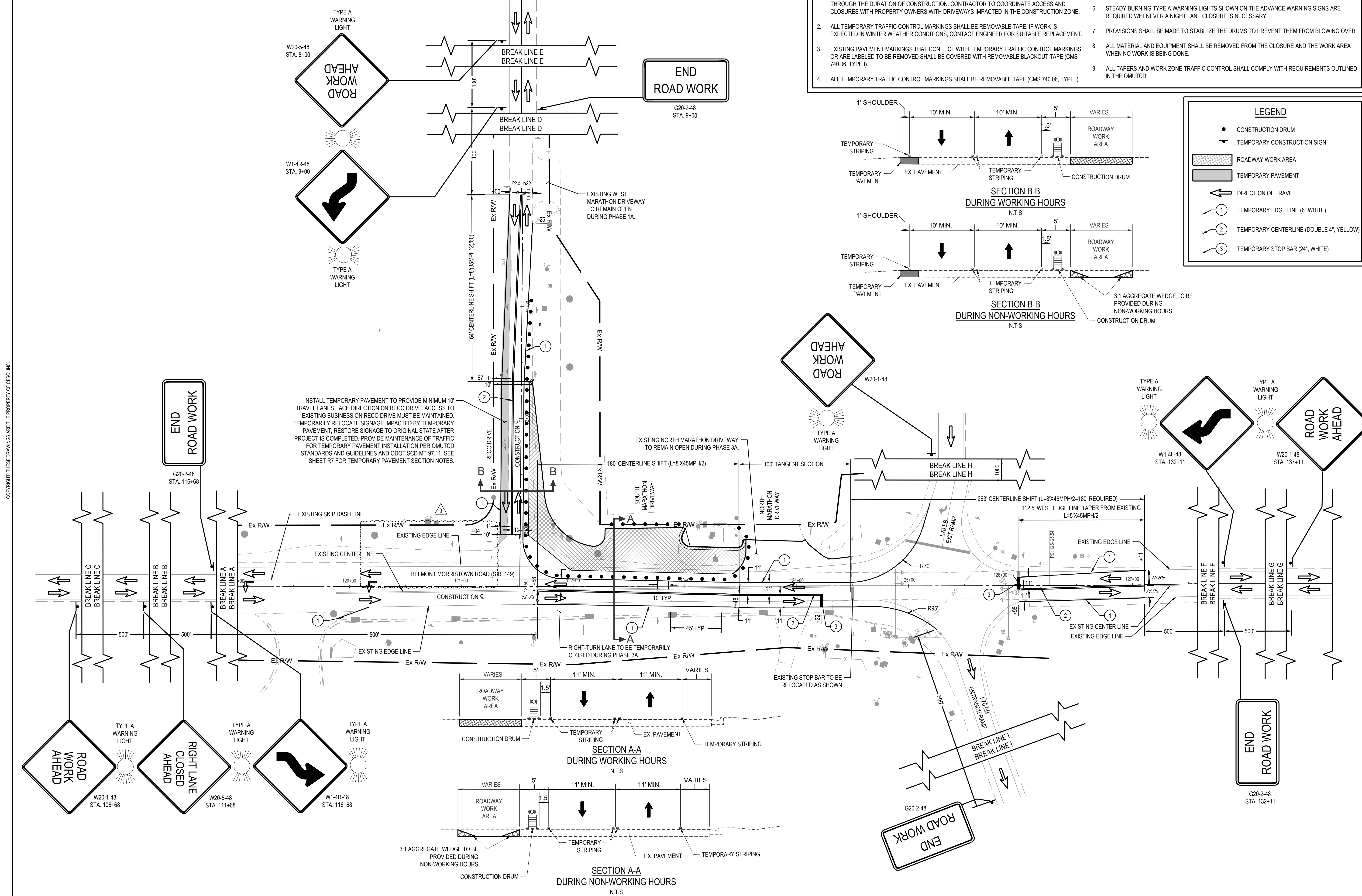


CALCULATED P.I.D. CHECKED TDH

MAINTENANCE OF TRAFFIC - PHASE 3A
S.R. 149

BEL-149-23.44

R10 R32

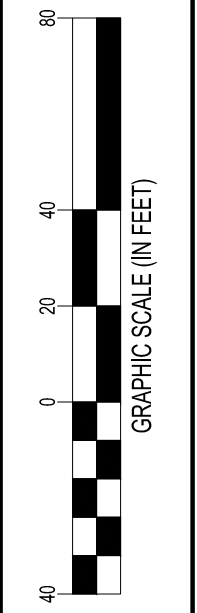
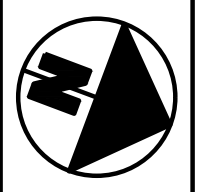
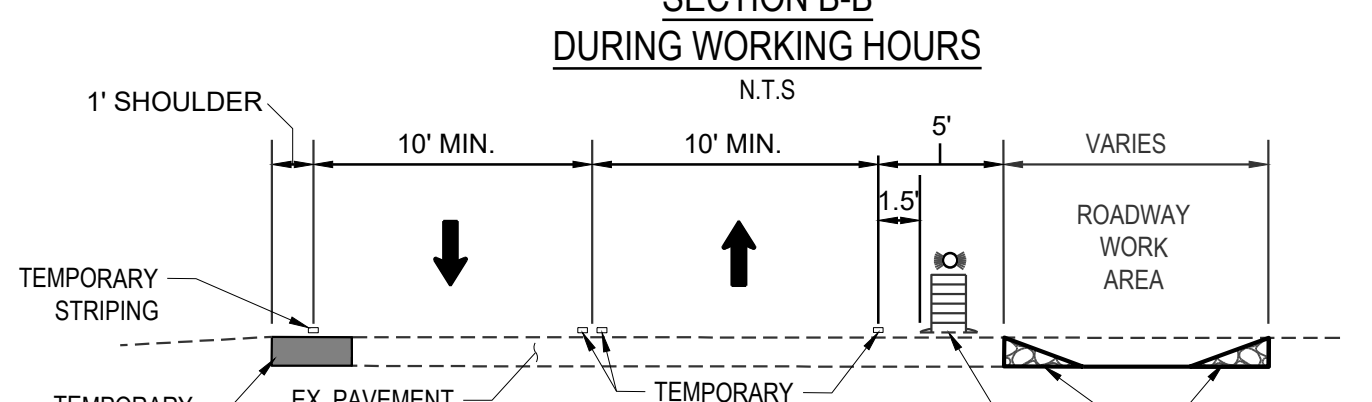
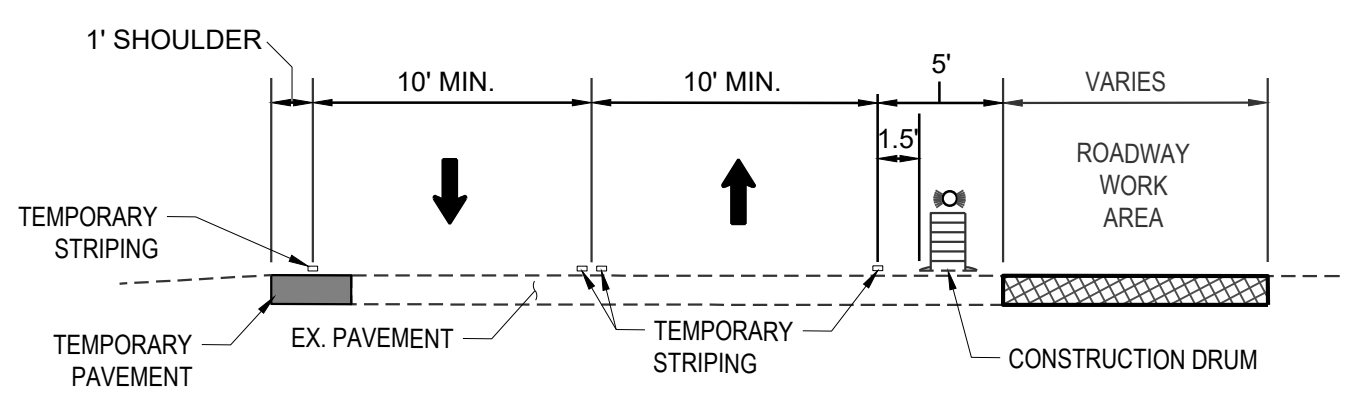
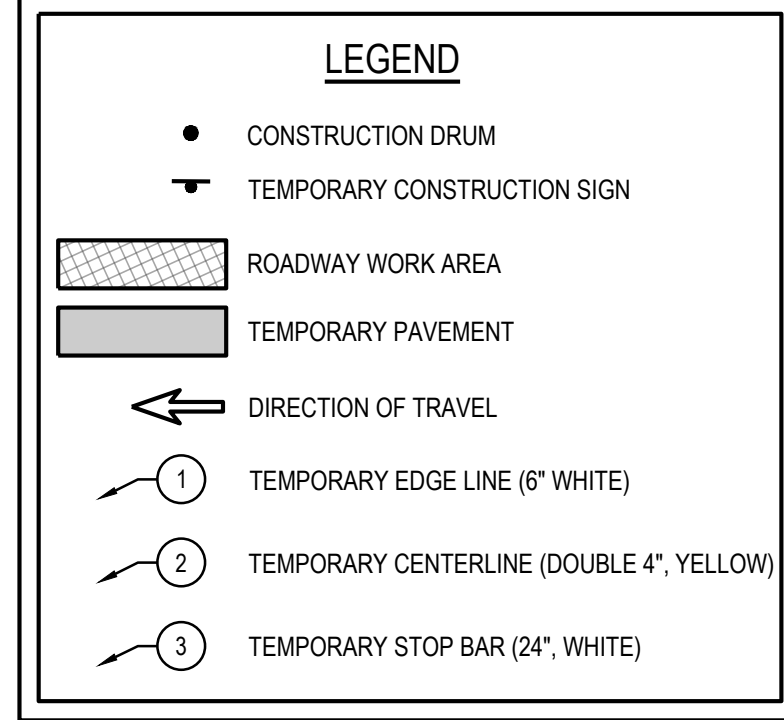


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9

W:\PROJECTS\LOVES\760267 - ST. CLAIRSVILLE, OH - SR 149\03-CIVIL\PLAN\ROADWAY\PL01760267_MP001A.DWG - 8/8/2024 11:28:30 AM

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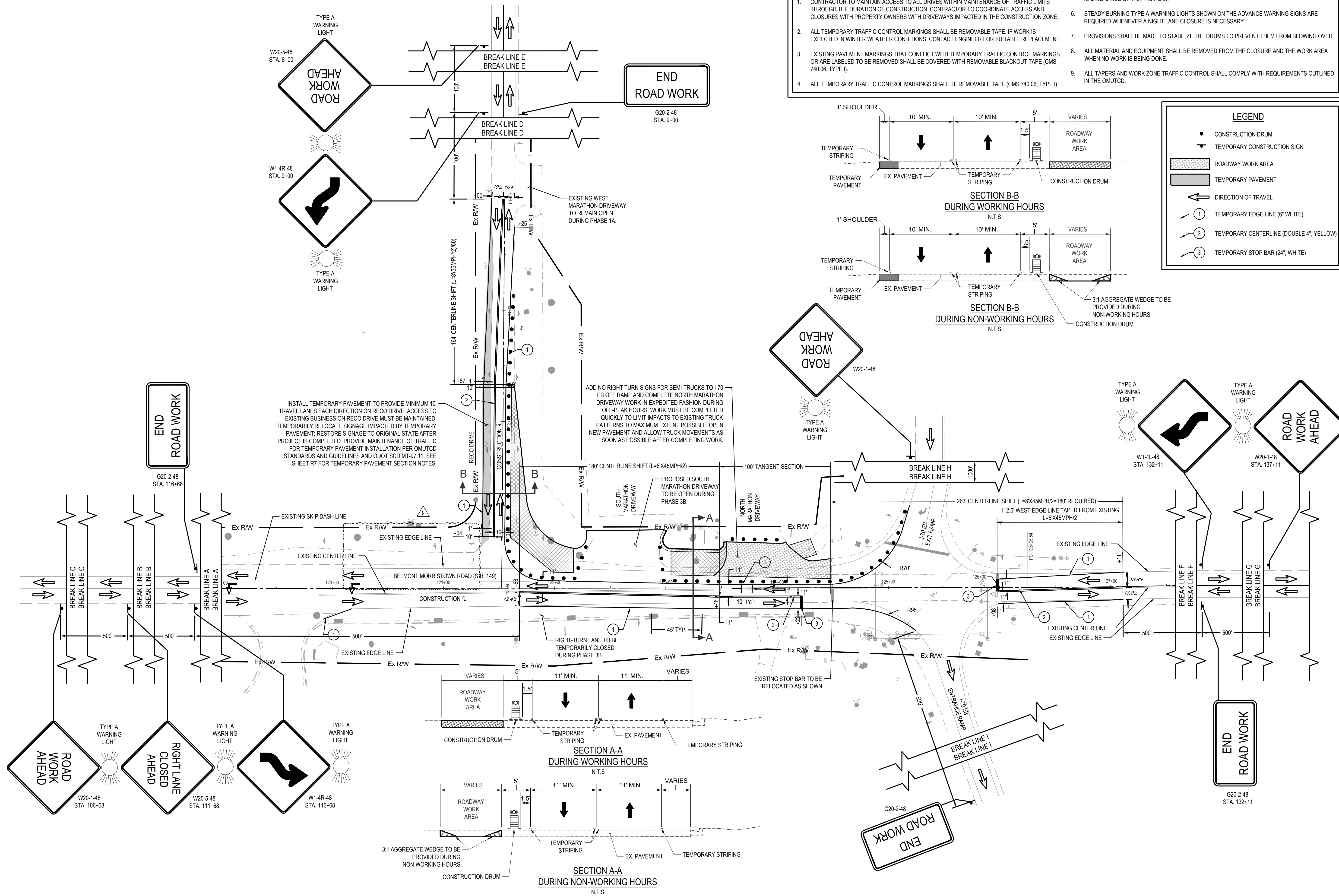
CALCULATED P.I.D. CHECKED TDH

MAINTENANCE OF TRAFFIC - PHASE 3B
S.R. 149

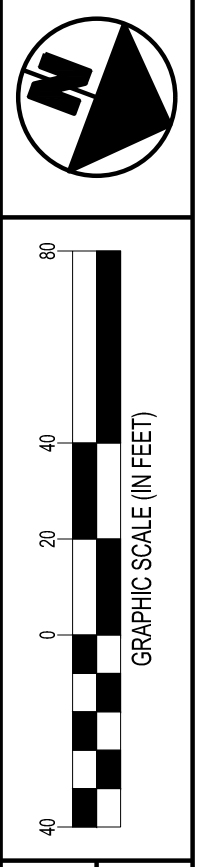
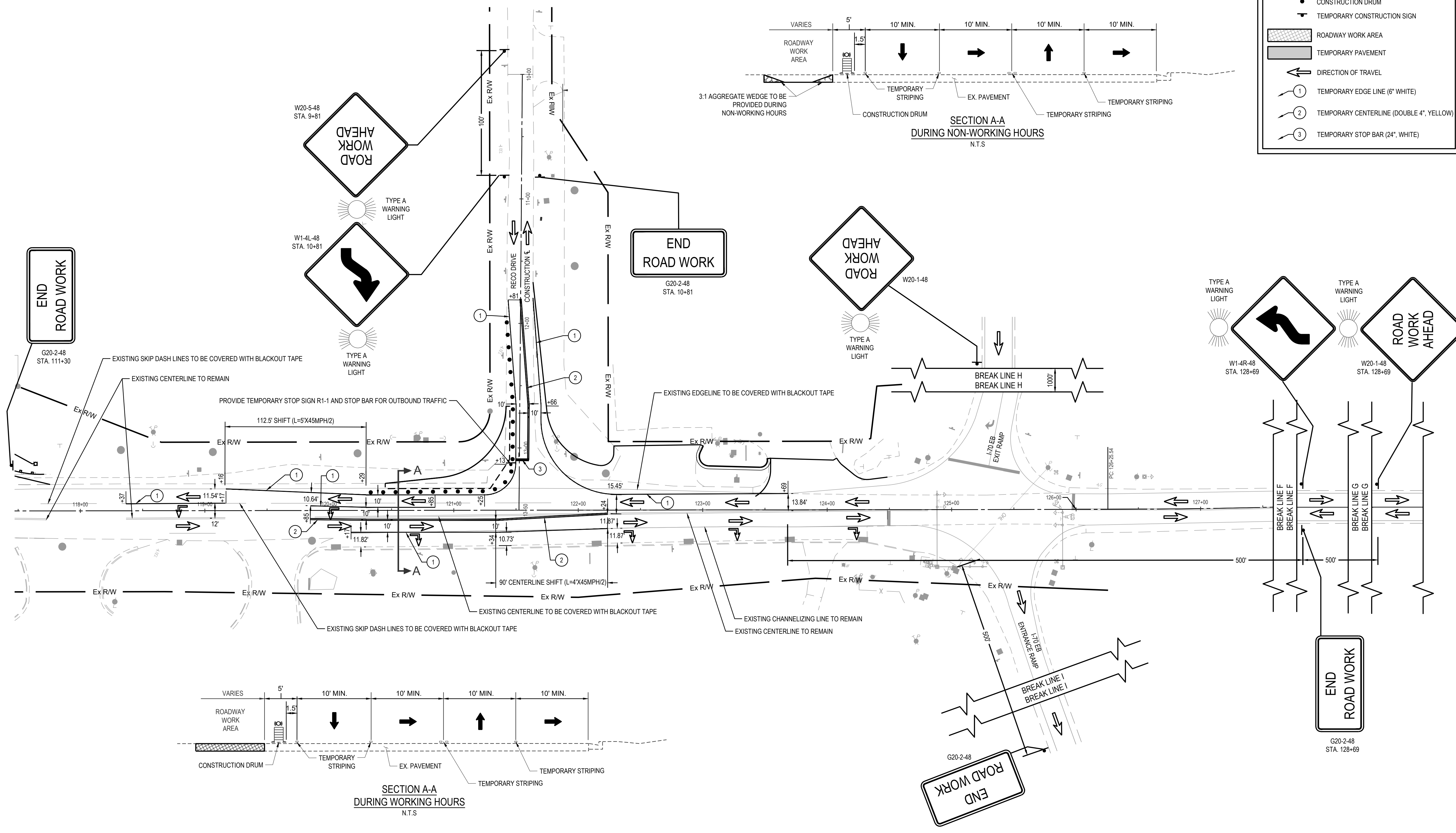
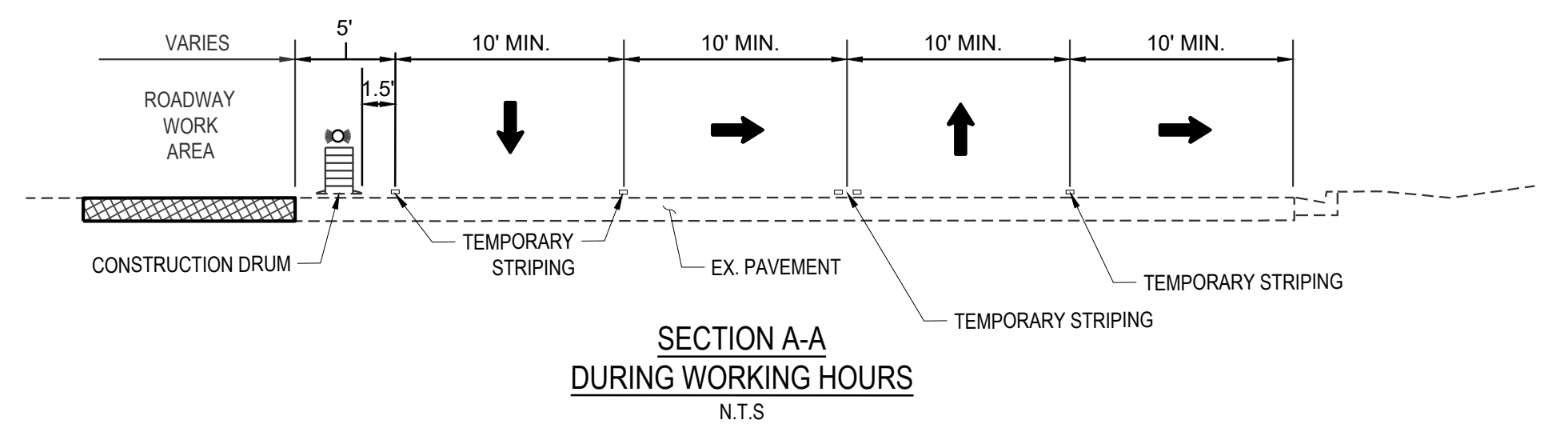
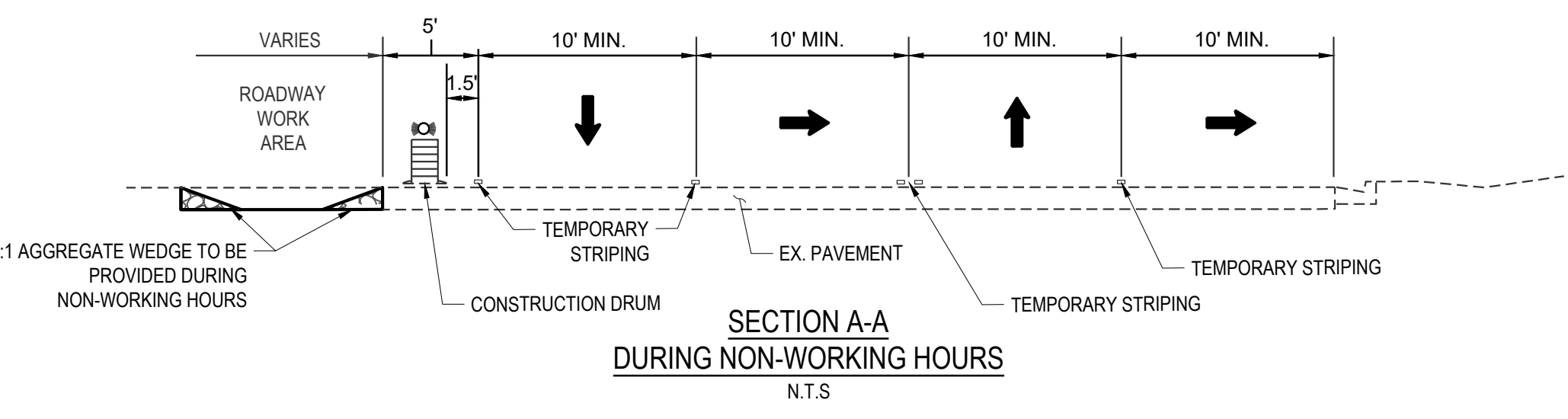
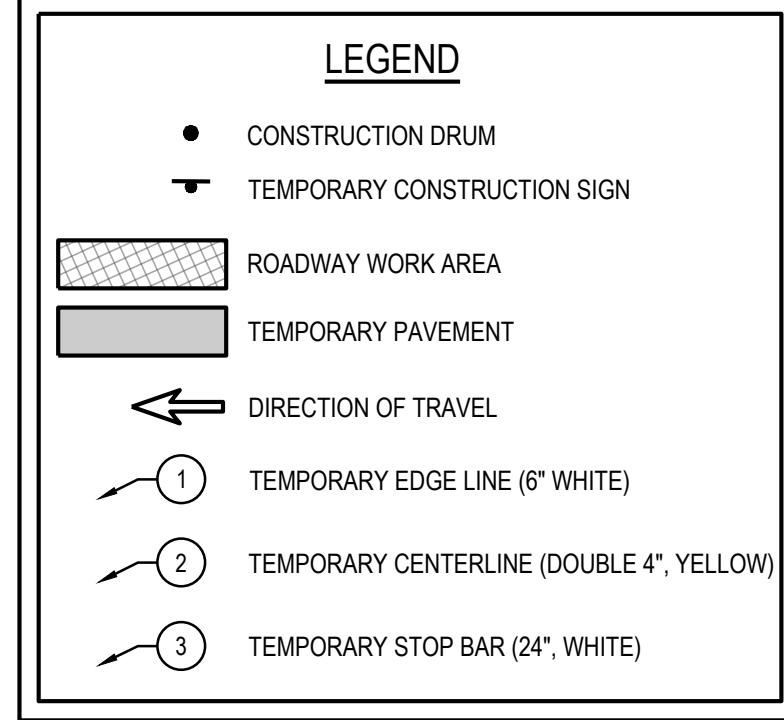
BEL-149-23.44

R11
R32

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CALCULATED P.I.D. CHECKED TDH

MAINTENANCE OF TRAFFIC - PHASE 3C
S.R. 149

BEL-149-23.44

R12
R32

SITE DESCRIPTION

CONSTRUCTION TO WIDEN EAST AND WEST SIDE OF S.R. 149 TO PROVIDE 175' NB LEFT TURN LANE INTO LOVE'S DRIVEWAY, 175' SB RIGHT-TURN LANE INTO LOVE'S DRIVEWAY, 164' TWLTL BETWEEN SOUTH PILOT DRIVEWAY & NORTH PILOT DRIVEWAY, 125' NB LEFT TURN LANE INTO RECO DRIVE, 200' SB RIGHT TURN LANE INTO RECO DRIVE AND ASSOCIATED LANE RECONFIGURATION TO ACCOMMODATE FOREMENTIONED IMPROVEMENTS. CONSTRUCTION TO WIDEN RECO DRIVE AND INSTALLATION OF TRAFFIC SIGNAL AT S.R. 149 & RECO DRIVE INTERSECTION. DITCH AND SLOPE GRADING AND INSTALLATION OF NECESSARY TRAFFIC CONTROL DEVICES, STORMWATER STRUCTURE AND CONDUIT MODIFICATIONS BETWEEN SOUTH MARATHON DRIVEWAY AND NORTH MARATHON DRIVEWAY.

NOI AND BMP NOTE

NOI AND BMP'S FOR ROADWAY WORK ARE INCLUDED WITH OVERALL SITE SUBMISSION FOR LOVE'S TRAVEL STOP DEVELOPMENT.

EARTH DISTURBED AREAS AND TOTAL AREA OF PROJECT

TOTAL AREA OF PROJECT: 2.07 ACRES
 PROJECT EARTH DISTURBING ACTIVITIES AREA: 1.77 ACRES
 CONTRACTOR EARTH DISTURBING ACTIVITIES AREA: 0.30 ACRES
 NOI EARTH DISTURBING ACTIVITIES AREA: 2.07 ACRES

SITE LOCATION

LAT = N 40° 03' 27"
 LONG = W 81° 03' 09"

REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 9-6-2024 FOR SPECIAL REINFORCED SOIL SLOPE (RSS) DESIGN FROM STA. 112+80 TO STA. 113+33.

REFER TO SITE PLANS FOR SITE GRADING, ROADWAY CONTRACTOR TO COORDINATE GRADING TIE IN AND ON SITE SWPPP MEASURES WITH SITE CONTRACTOR

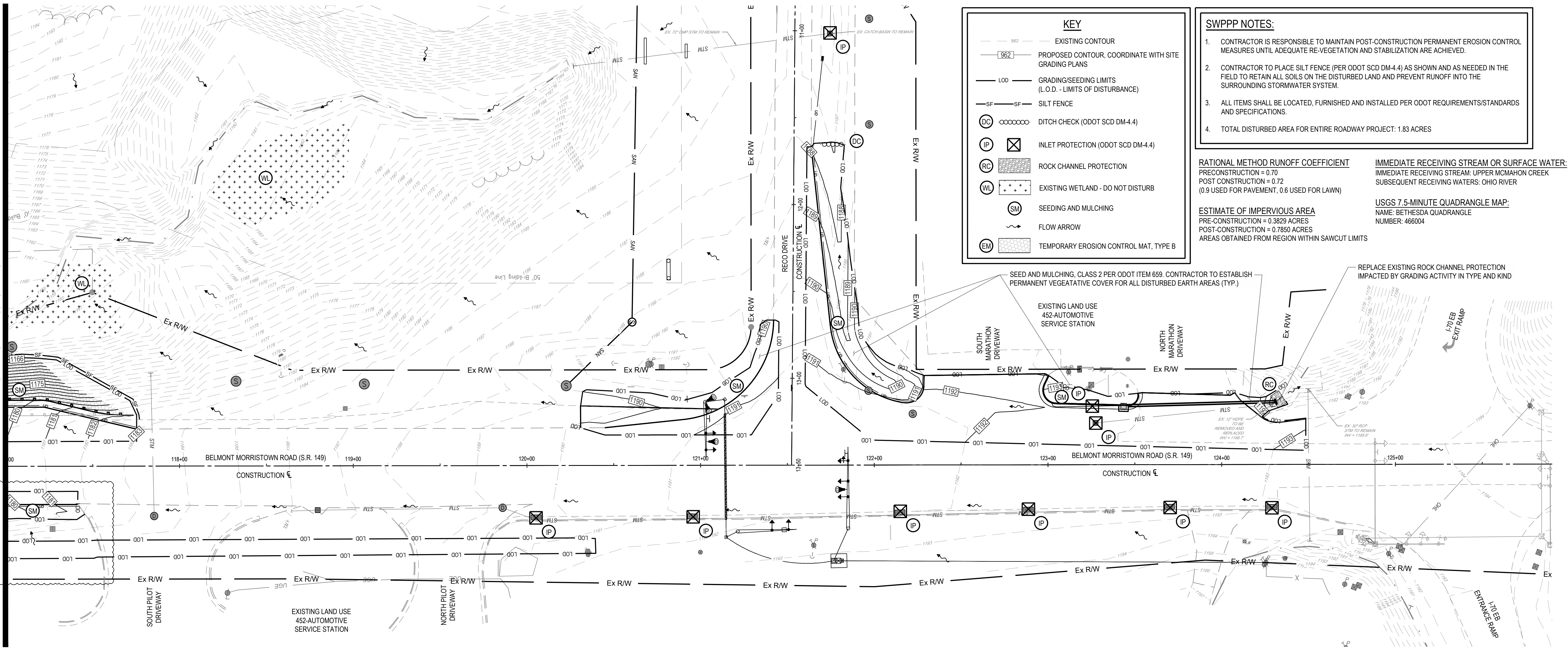
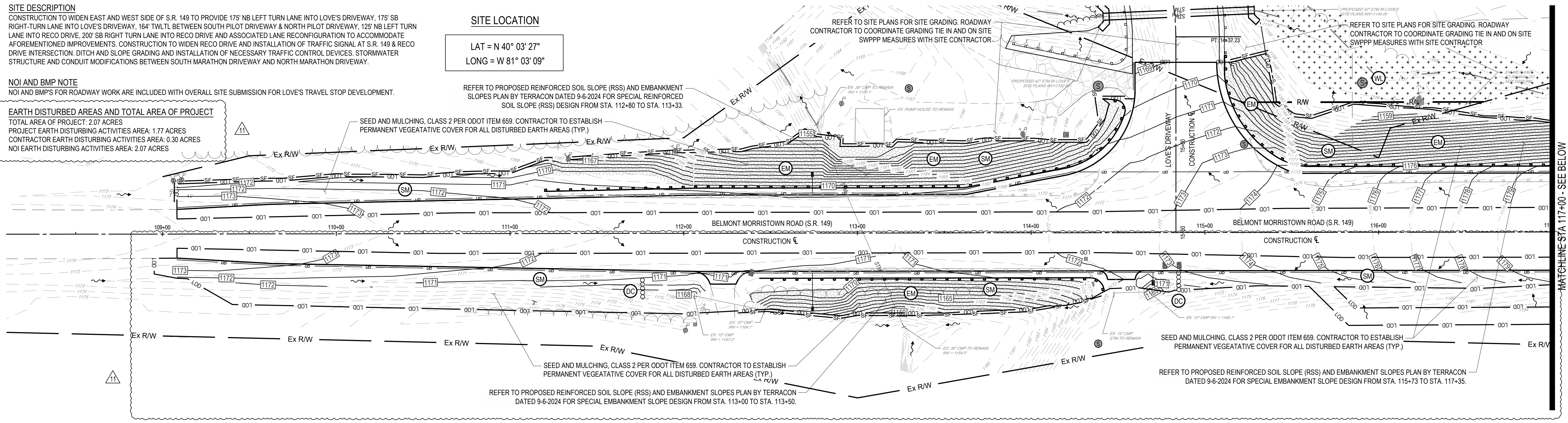
REFER TO SITE PLANS FOR SITE GRADING, ROADWAY CONTRACTOR TO COORDINATE GRADING TIE IN AND ON SITE SWPPP MEASURES WITH SITE CONTRACTOR

SEED AND MULCHING, CLASS 2 PER ODOT ITEM 659. CONTRACTOR TO ESTABLISH PERMANENT VEGETATIVE COVER FOR ALL DISTURBED EARTH AREAS (TYP.)

SEED AND MULCHING, CLASS 2 PER ODOT ITEM 659. CONTRACTOR TO ESTABLISH PERMANENT VEGETATIVE COVER FOR ALL DISTURBED EARTH AREAS (TYP.)

SEED AND MULCHING, CLASS 2 PER ODOT ITEM 659. CONTRACTOR TO ESTABLISH PERMANENT VEGETATIVE COVER FOR ALL DISTURBED EARTH AREAS (TYP.)

REFER TO PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES PLAN BY TERRACON DATED 9-6-2024 FOR SPECIAL EMBANKMENT SLOPE DESIGN FROM STA. 115+73 TO STA. 117+35.



KEY

- EXISTING CONTOUR
- PROPOSED CONTOUR, COORDINATE WITH SITE GRADING PLANS
- GRADING/SEEDING LIMITS (L.O.D. - LIMITS OF DISTURBANCE)
- SILT FENCE
- DITCH CHECK (ODOT SCD DM-4.4)
- INLET PROTECTION (ODOT SCD DM-4.4)
- ROCK CHANNEL PROTECTION
- EXISTING WETLAND - DO NOT DISTURB
- SEEDING AND MULCHING
- FLOW ARROW
- TEMPORARY EROSION CONTROL MAT, TYPE B

SWPPP NOTES:

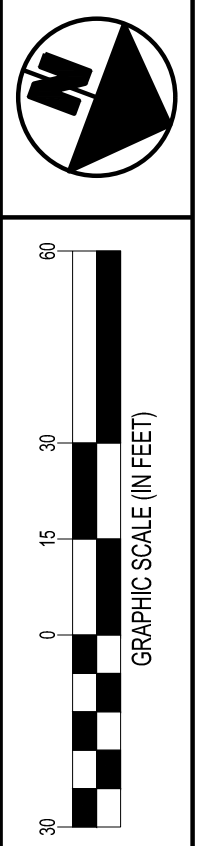
- CONTRACTOR IS RESPONSIBLE TO MAINTAIN POST-CONSTRUCTION PERMANENT EROSION CONTROL MEASURES UNTIL ADEQUATE RE-VEGETATION AND STABILIZATION ARE ACHIEVED.
- CONTRACTOR TO PLACE SILT FENCE (PER ODOT SCD DM-4.4) AS SHOWN AND AS NEEDED IN THE FIELD TO RETAIN ALL SOILS ON THE DISTURBED LAND AND PREVENT RUNOFF INTO THE SURROUNDING STORMWATER SYSTEM.
- ALL ITEMS SHALL BE LOCATED, FURNISHED AND INSTALLED PER ODOT REQUIREMENTS/STANDARDS AND SPECIFICATIONS.
- TOTAL DISTURBED AREA FOR ENTIRE ROADWAY PROJECT: 1.83 ACRES

RATIONAL METHOD RUNOFF COEFFICIENT
 PRECONSTRUCTION = 0.70
 POST CONSTRUCTION = 0.72
 (0.9 USED FOR PAVEMENT, 0.6 USED FOR LAWN)

IMMEDIATE RECEIVING STREAM OR SURFACE WATER:
 IMMEDIATE RECEIVING STREAM: UPPER MCMAHON CREEK
 SUBSEQUENT RECEIVING WATERS: OHIO RIVER

USGS 7.5-MINUTE QUADRANGLE MAP:
 NAME: BETHESDA QUADRANGLE
 NUMBER: 466004

ESTIMATE OF IMPERVIOUS AREA
 PRE-CONSTRUCTION = 0.3829 ACRES
 POST-CONSTRUCTION = 0.7850 ACRES
 AREAS OBTAINED FROM REGION WITHIN SAWCUT LIMITS

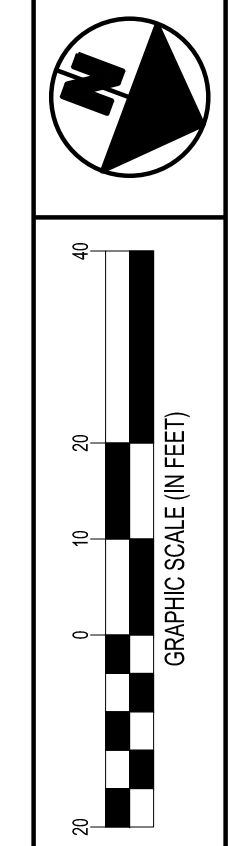


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SITE PLAN
 S.R. 149

BEL-149-23.44

R13
 R32

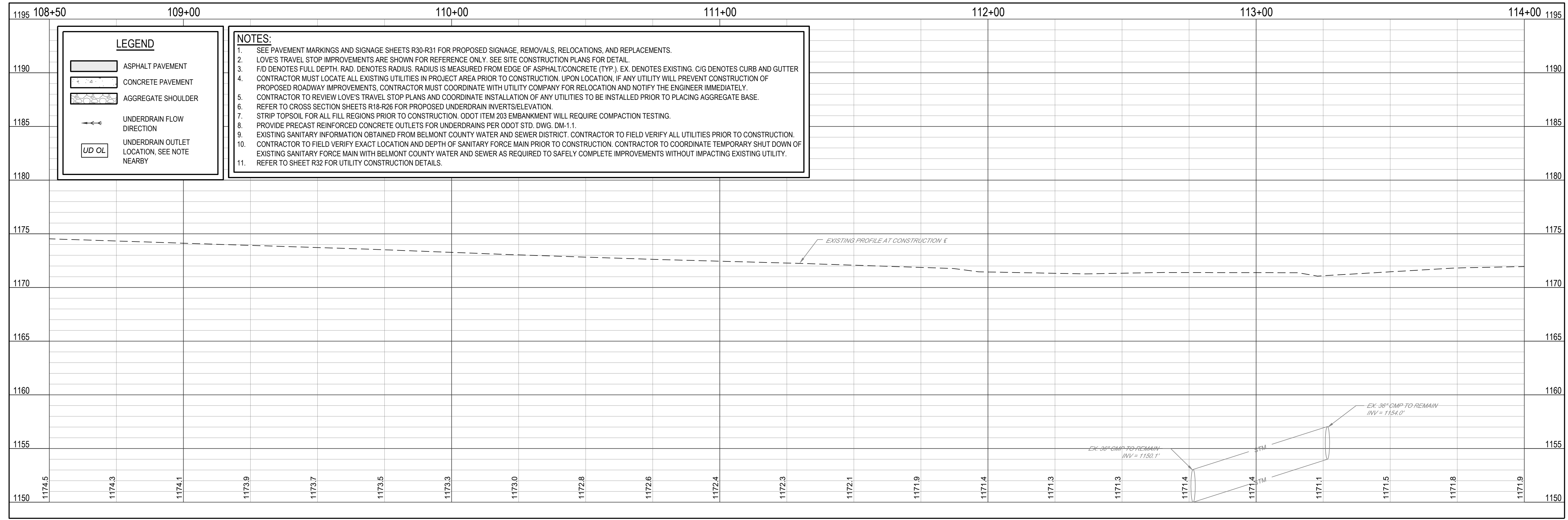
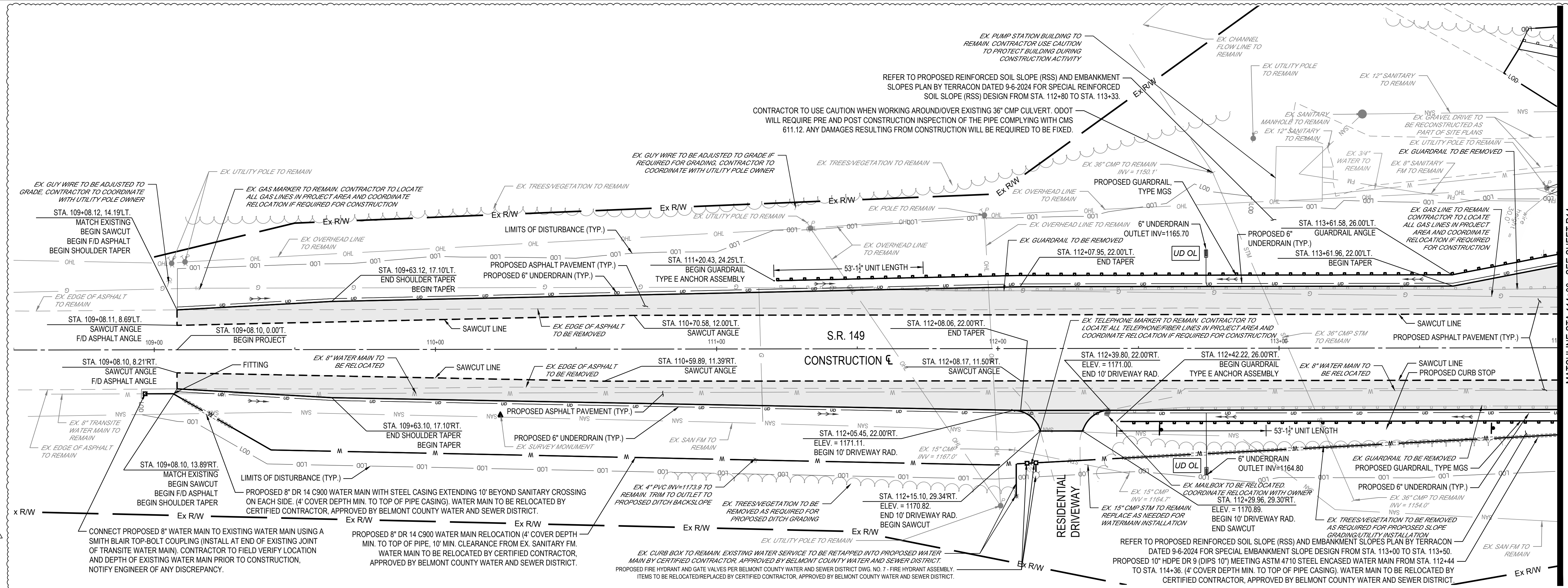


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P.D.
CHECKED
TDH

PLAN & PROFILE - STA. 108+50 TO STA. 114+00
S.R. 149

BEL-149-23.44

R14
R32

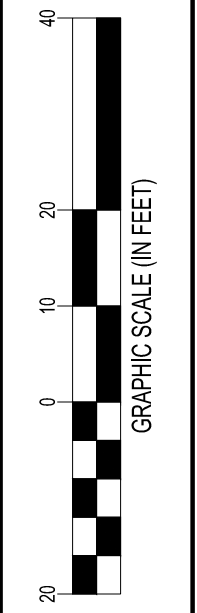
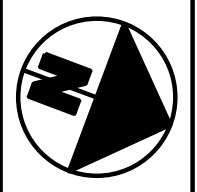


LEGEND

	ASPHALT PAVEMENT
	CONCRETE PAVEMENT
	AGGREGATE SHOULDER
	UNDERDRAIN FLOW DIRECTION
	UNDERDRAIN OUTLET LOCATION, SEE NOTE NEARBY

- NOTES:**
- SEE PAVEMENT MARKINGS AND SIGNAGE SHEETS R30-R31 FOR PROPOSED SIGNAGE, REMOVALS, RELOCATIONS, AND REPLACEMENTS.
 - LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
 - F/D DENOTES FULL DEPTH, RAD. DENOTES RADIUS, RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.), EX. DENOTES EXISTING, C/G DENOTES CURB AND GUTTER.
 - CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.
 - CONTRACTOR TO REVIEW LOVE'S TRAVEL STOP PLANS AND COORDINATE INSTALLATION OF ANY UTILITIES TO BE INSTALLED PRIOR TO PLACING AGGREGATE BASE.
 - REFER TO CROSS SECTION SHEETS R18-R26 FOR PROPOSED UNDERDRAIN INVERTS/ELEVATION.
 - STRIP TOPSOIL FOR ALL FILL REGIONS PRIOR TO CONSTRUCTION. ODOT ITEM 203 EMBANKMENT WILL REQUIRE COMPACTION TESTING.
 - PROVIDE PRECAST REINFORCED CONCRETE OUTLETS FOR UNDERDRAINS PER ODOT STD. DWG. DM-1.1.
 - EXISTING SANITARY INFORMATION OBTAINED FROM BELMONT COUNTY WATER AND SEWER DISTRICT. CONTRACTOR TO FIELD VERIFY ALL UTILITIES PRIOR TO CONSTRUCTION.
 - CONTRACTOR TO FIELD VERIFY EXACT LOCATION AND DEPTH OF SANITARY FORCE MAIN PRIOR TO CONSTRUCTION. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING SANITARY FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER AS REQUIRED TO SAFELY COMPLETE IMPROVEMENTS WITHOUT IMPACTING EXISTING UTILITY.
 - REFER TO SHEET R32 FOR UTILITY CONSTRUCTION DETAILS.

STATE ROUTE 149 PROFILE
Scale: 1" = 20' Horiz.; 1" = 5' Vert.

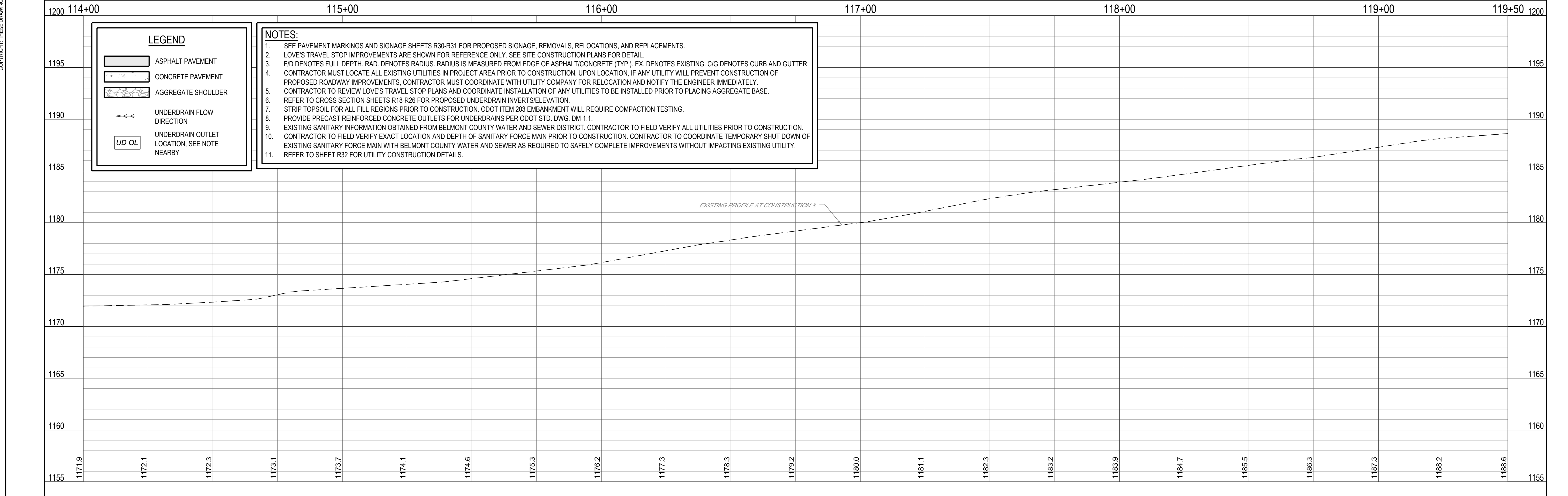
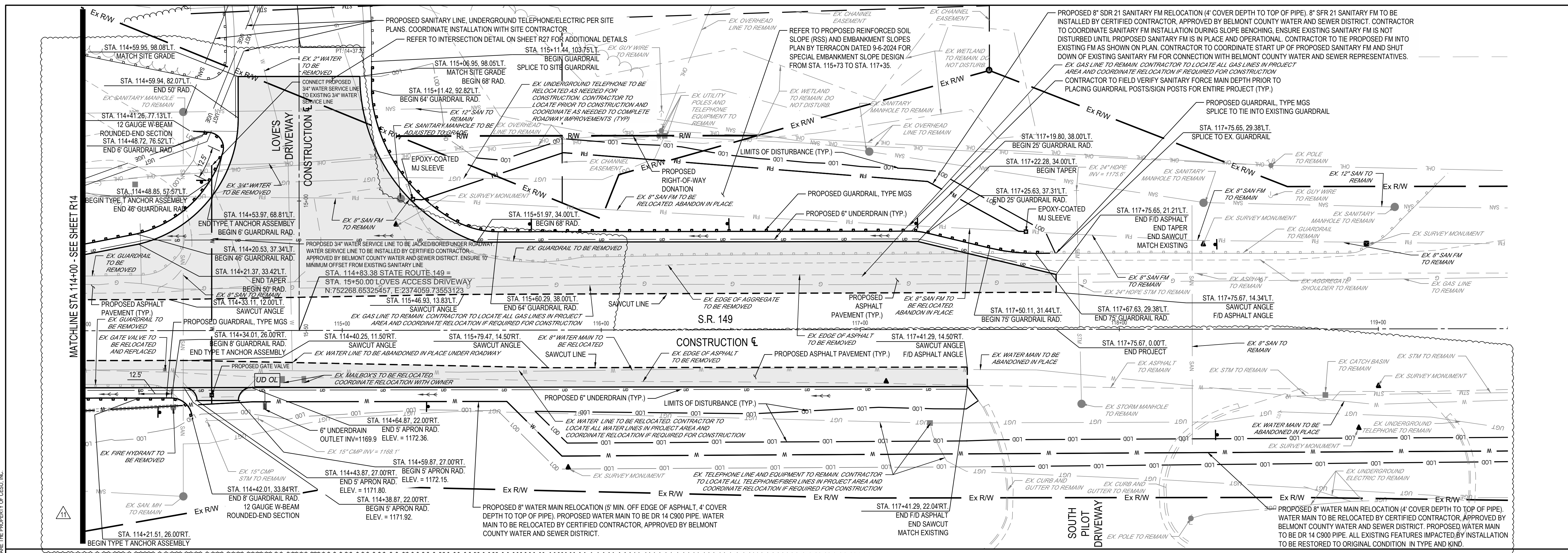


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PLAN & PROFILE - STA. 114+00 TO STA. 119+50
S.R. 149

BEL-149-23.44

R15
R32

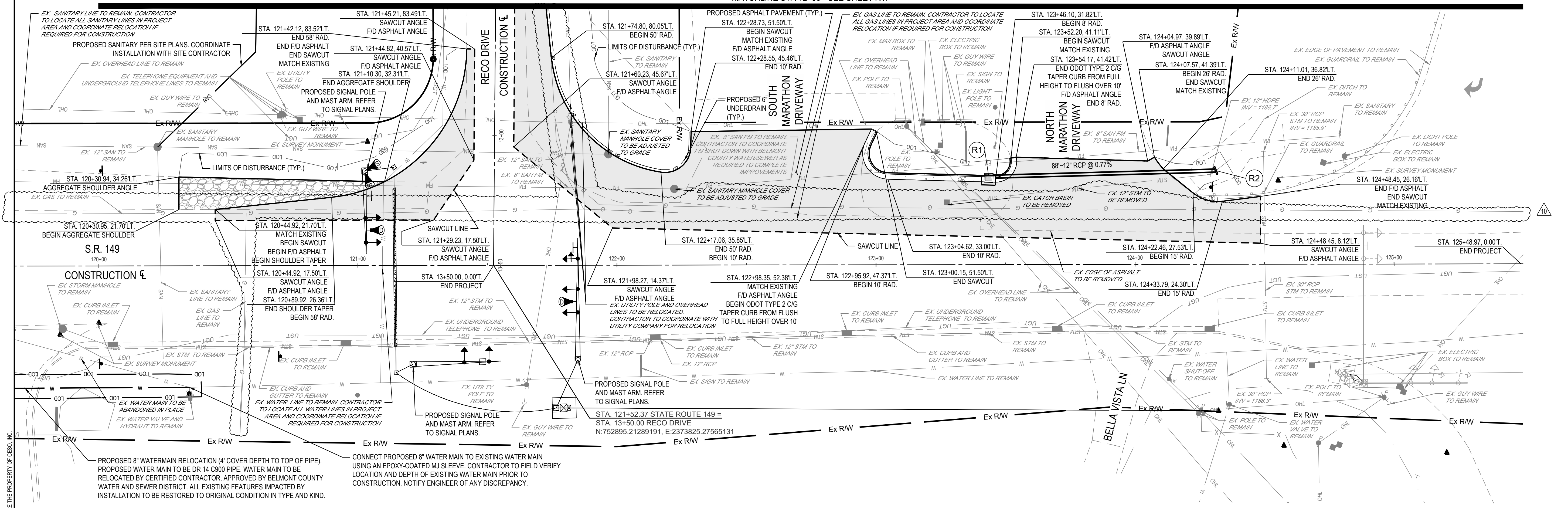


STATE ROUTE 149 PROFILE
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MATCHLINE STA 12+50 - SEE SHEET R17



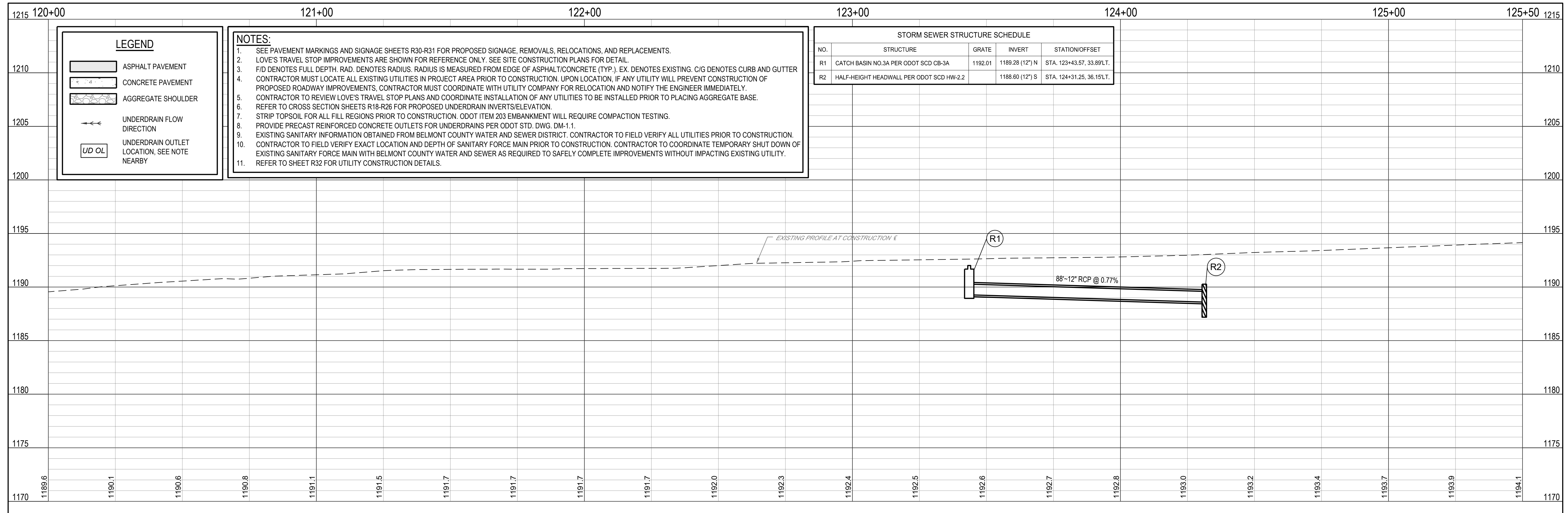
LEGEND

	ASPHALT PAVEMENT
	CONCRETE PAVEMENT
	AGGREGATE SHOULDER
	UNDERDRAIN FLOW DIRECTION
	UNDERDRAIN OUTLET LOCATION. SEE NOTE NEARBY

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 - CONTRACTOR TO REVIEW LOVE'S TRAVEL STOP PLANS AND COORDINATE INSTALLATION OF ANY UTILITIES TO BE INSTALLED PRIOR TO PLACING AGGREGATE BASE.
 - REFER TO CROSS SECTION SHEETS R18-R26 FOR PROPOSED UNDERDRAIN INVERTS/ELEVATION.
 - STRIP TOPSOIL FOR ALL FILL REGIONS PRIOR TO CONSTRUCTION. ODOT ITEM 203 EMBANKMENT WILL REQUIRE COMPACTION TESTING.
 - PROVIDE PRECAST REINFORCED CONCRETE OUTLETS FOR UNDERDRAINS PER ODOT STD. DWG. DM-1.1.
 - EXISTING SANITARY INFORMATION OBTAINED FROM BELMONT COUNTY WATER AND SEWER DISTRICT. CONTRACTOR TO FIELD VERIFY ALL UTILITIES PRIOR TO CONSTRUCTION.
 - CONTRACTOR TO FIELD VERIFY EXACT LOCATION AND DEPTH OF SANITARY FORCE MAIN PRIOR TO CONSTRUCTION. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING SANITARY FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER AS REQUIRED TO SAFELY COMPLETE IMPROVEMENTS WITHOUT IMPACTING EXISTING UTILITY.
 - REFER TO SHEET R32 FOR UTILITY CONSTRUCTION DETAILS.

STORM SEWER STRUCTURE SCHEDULE

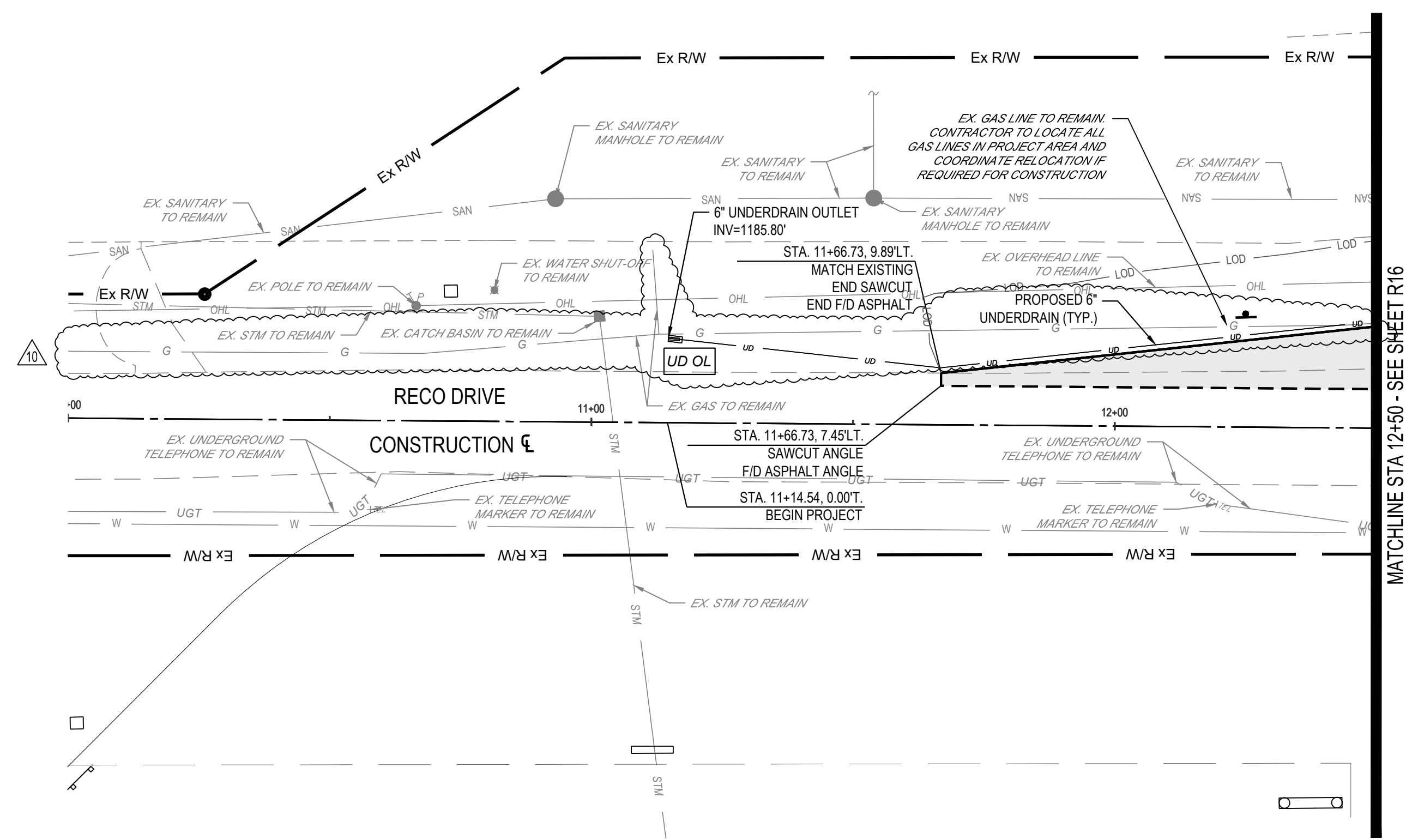
NO.	STRUCTURE	GRATE	INVERT	STATION/OFFSET
R1	CATCH BASIN NO.3A PER ODOT SCD CB-3A	1192.01	1189.28 (12") N	STA. 123+43.57, 33.89'L.
R2	HALF-HEIGHT HEADWALL PER ODOT SCD HW-2.2		1188.60 (12") S	STA. 124+31.25, 36.15'L.



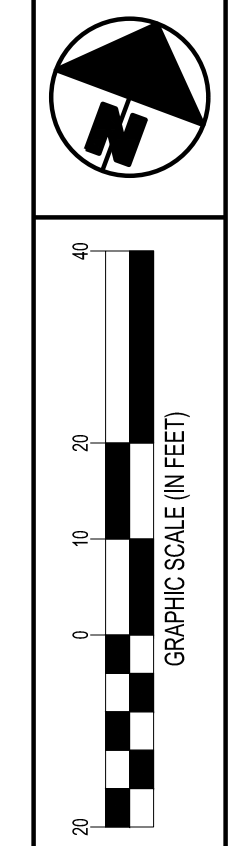
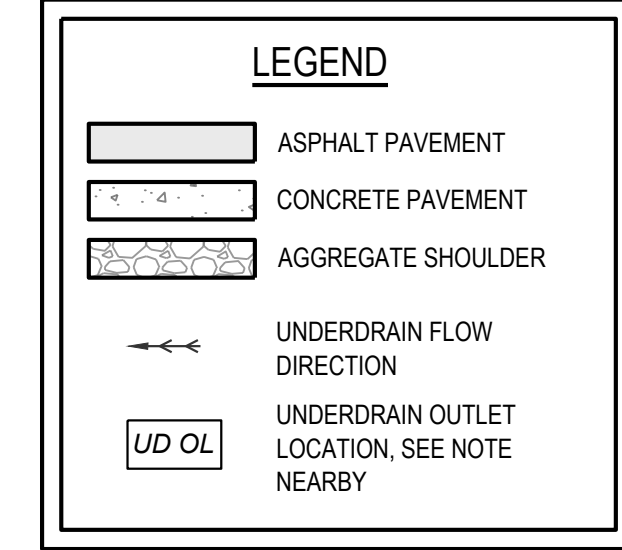
STATE ROUTE 149 PROFILE
Scale: 1" = 20' Horiz.; 1" = 5' Vert.

CALCULATED P.D. CHECKED TDH
 PLAN & PROFILE - STA. 120+00 TO STA. 125+50
 S.R. 149
 BEL-149-23.44

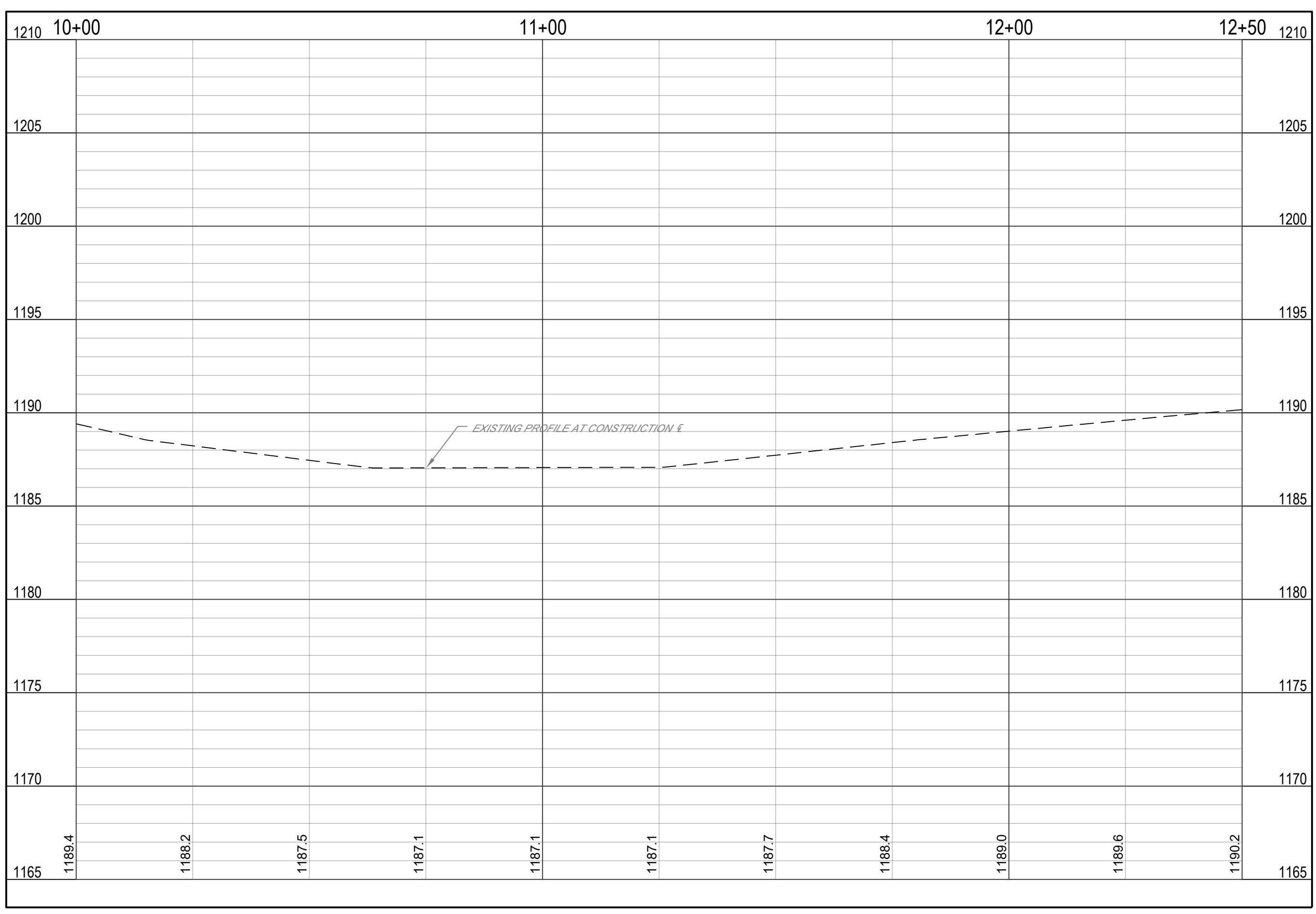
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- NOTES:**
- SEE PAVEMENT MARKINGS AND SIGNAGE SHEETS R30-R31 FOR PROPOSED SIGNAGE, REMOVALS, RELOCATIONS, AND REPLACEMENTS.
 - LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
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 - CONTRACTOR MUST LOCATE ALL EXISTING UTILITIES IN PROJECT AREA PRIOR TO CONSTRUCTION. UPON LOCATION, IF ANY UTILITY WILL PREVENT CONSTRUCTION OF PROPOSED ROADWAY IMPROVEMENTS, CONTRACTOR MUST COORDINATE WITH UTILITY COMPANY FOR RELOCATION AND NOTIFY THE ENGINEER IMMEDIATELY.
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 - REFER TO SHEET R32 FOR UTILITY CONSTRUCTION DETAILS.



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CHECKED
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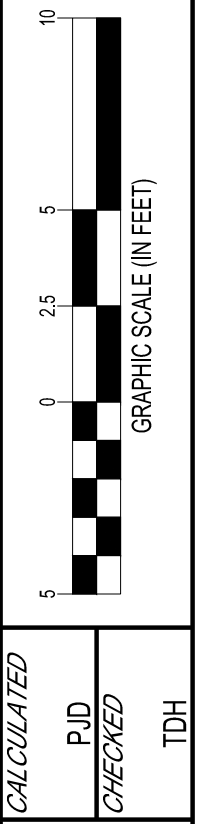
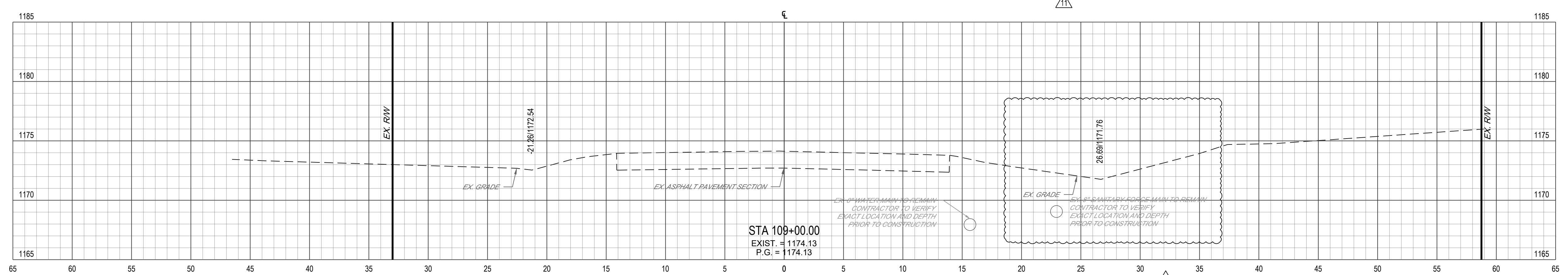
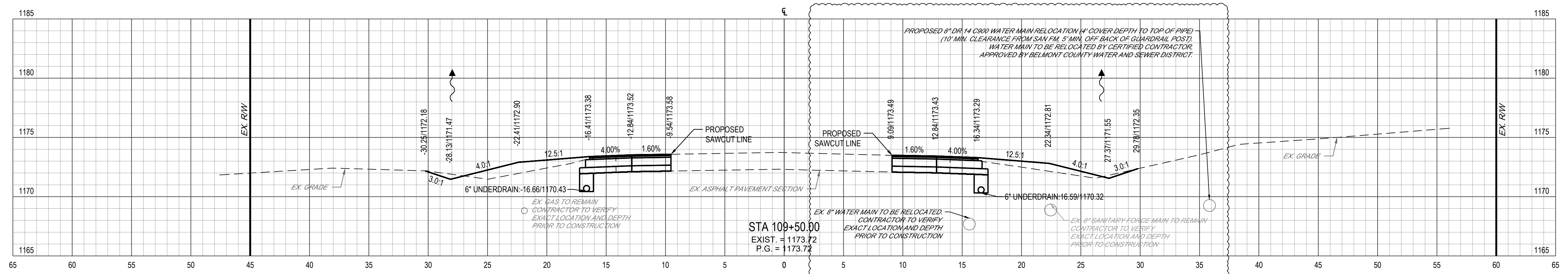
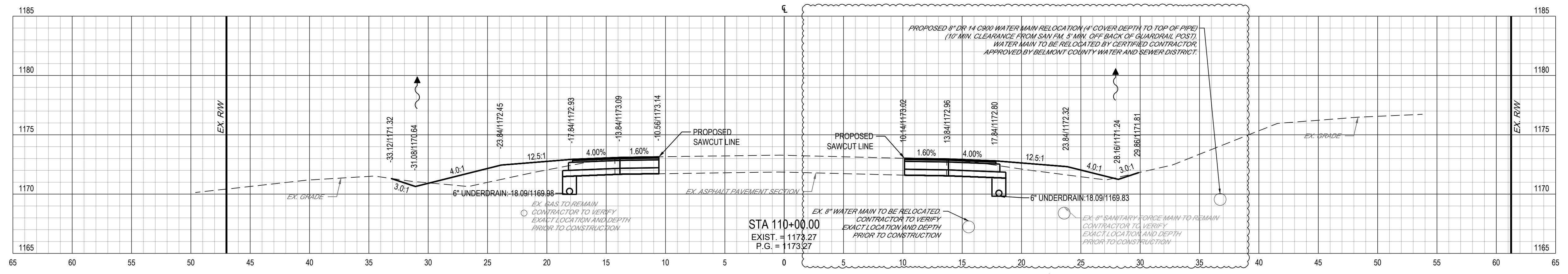
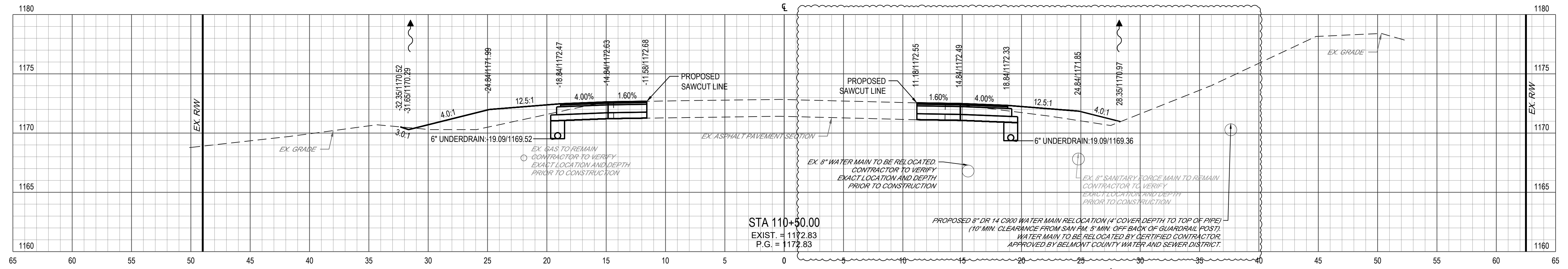


PLAN & PROFILE - RECO DRIVE
S.R. 149

BEL-149-23.44

R17
R32

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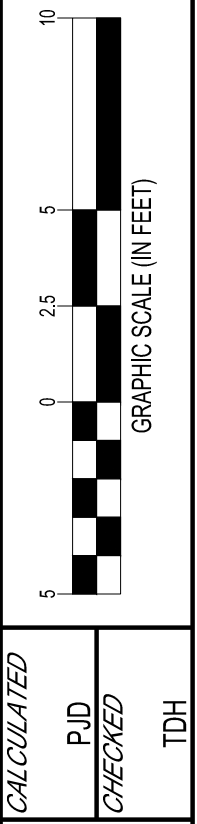
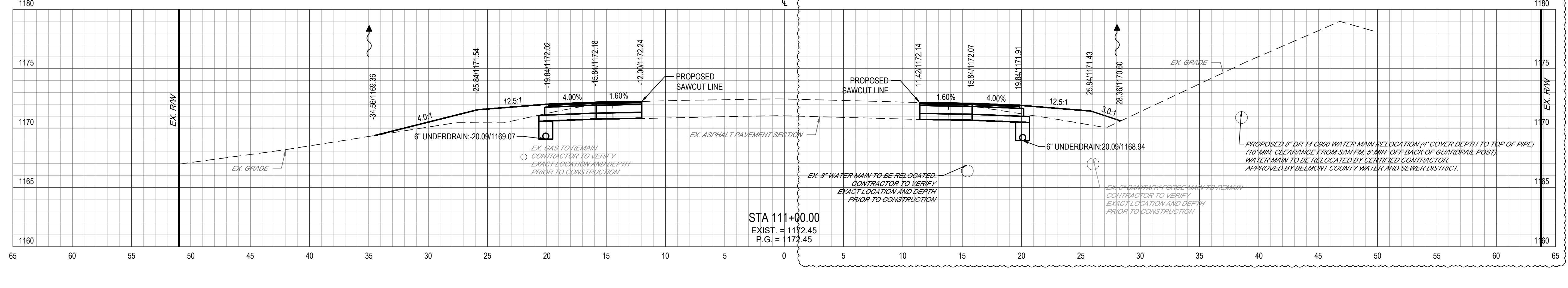
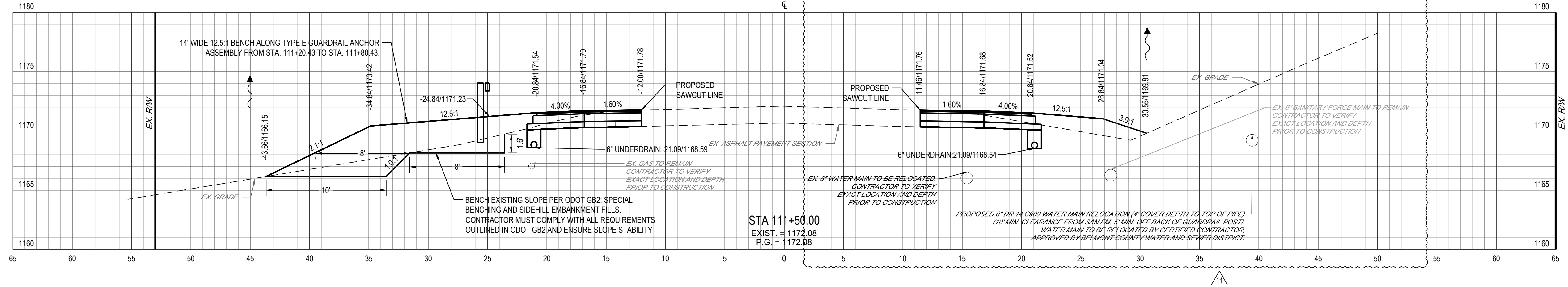
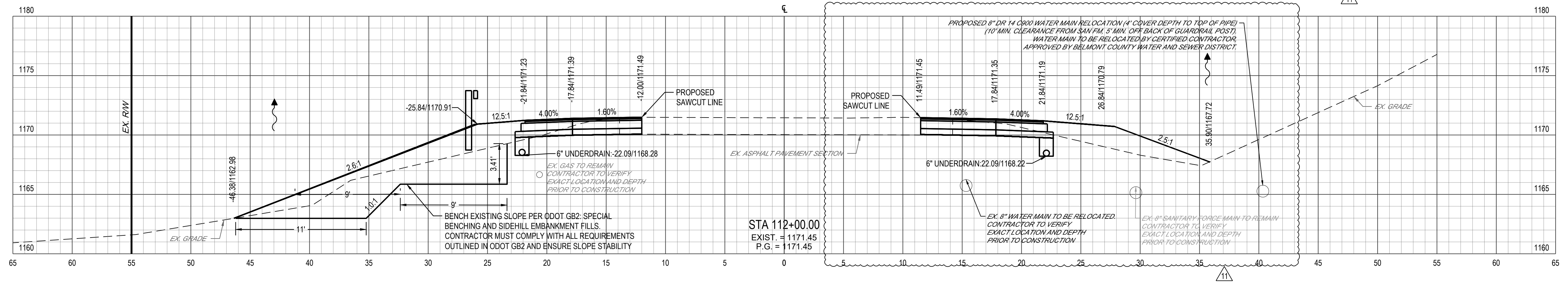
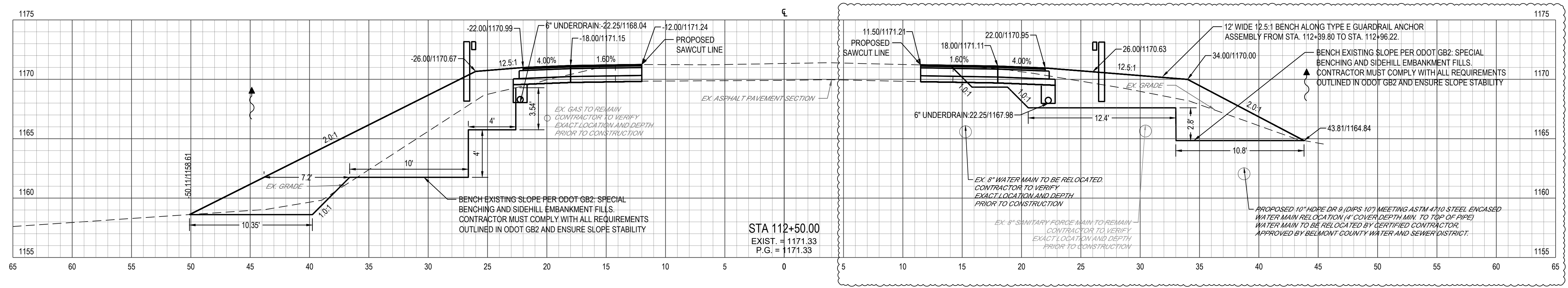


CROSS SECTIONS - STA. 109+00 TO STA. 110+50
 S.R. 149

BEL-149-23.44

R18
 R32

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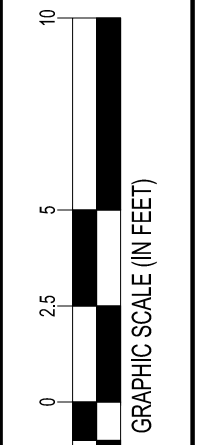
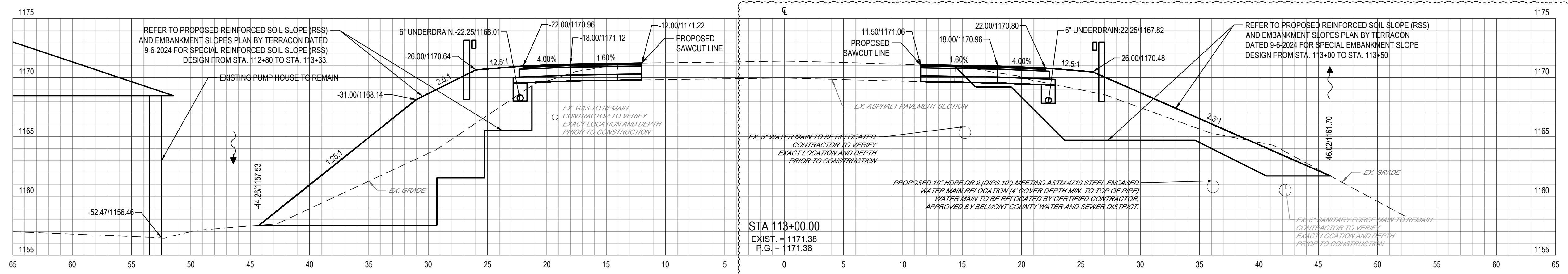
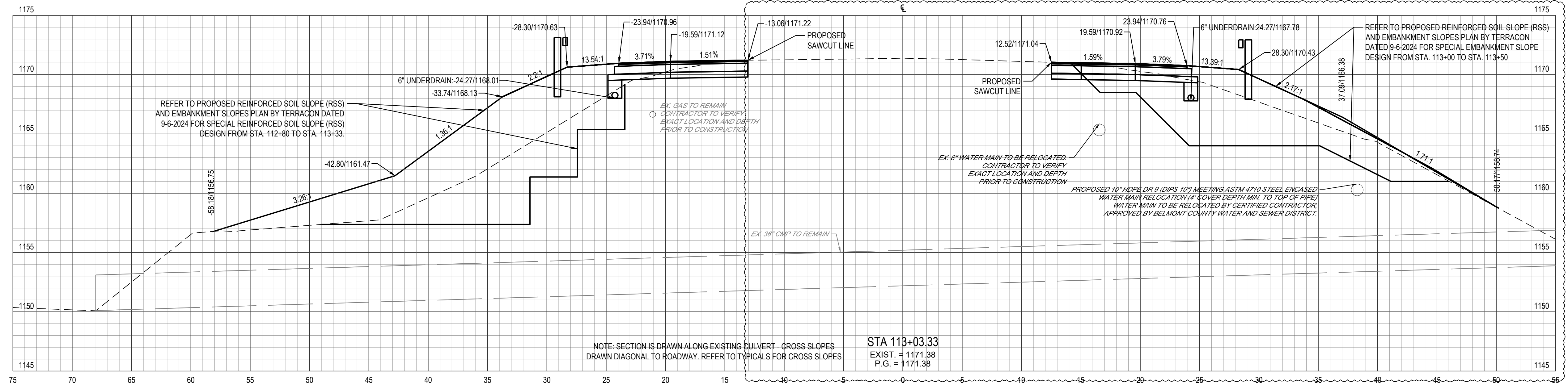
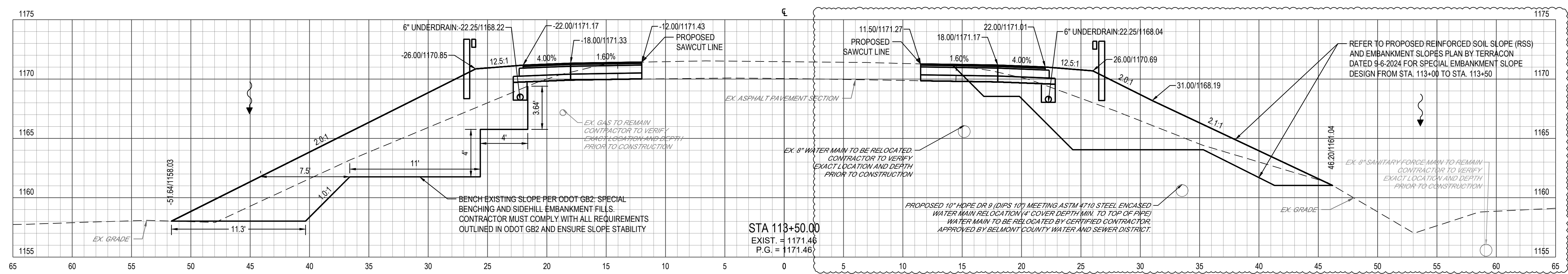
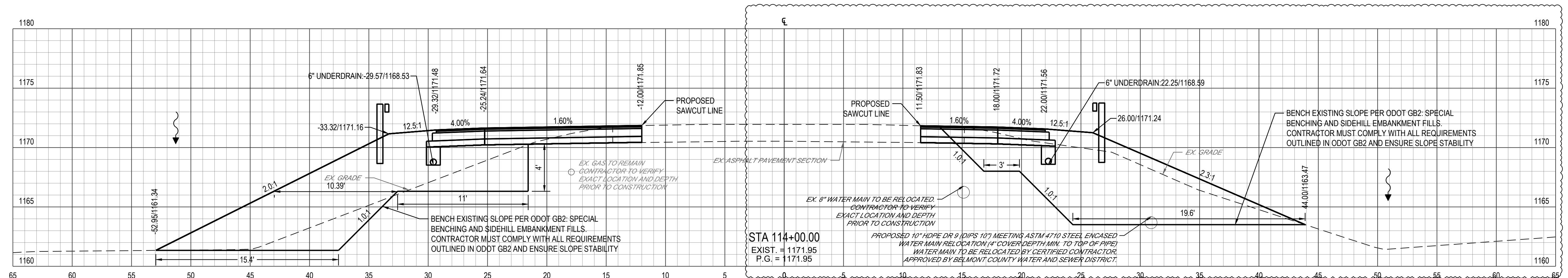
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CROSS SECTIONS - STA. 111+00 TO STA. 112+50
S.R. 149

BEL-149-23.44

R19
R32

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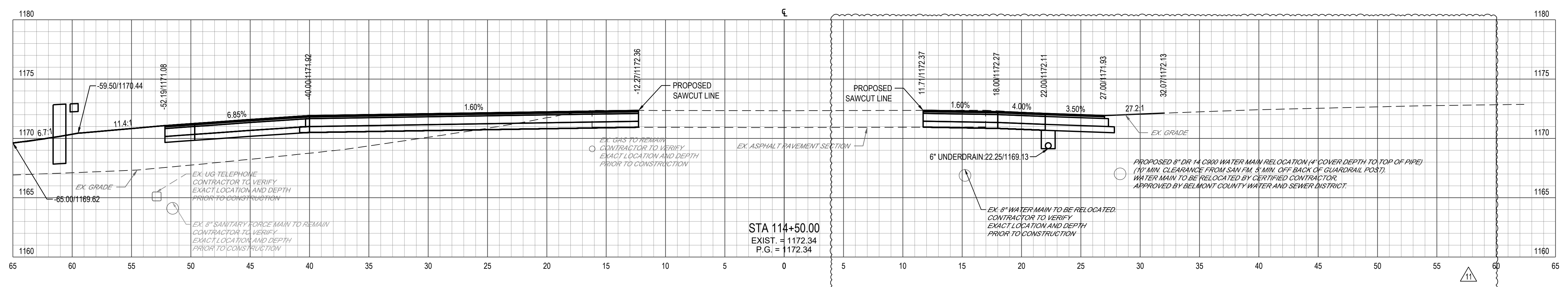
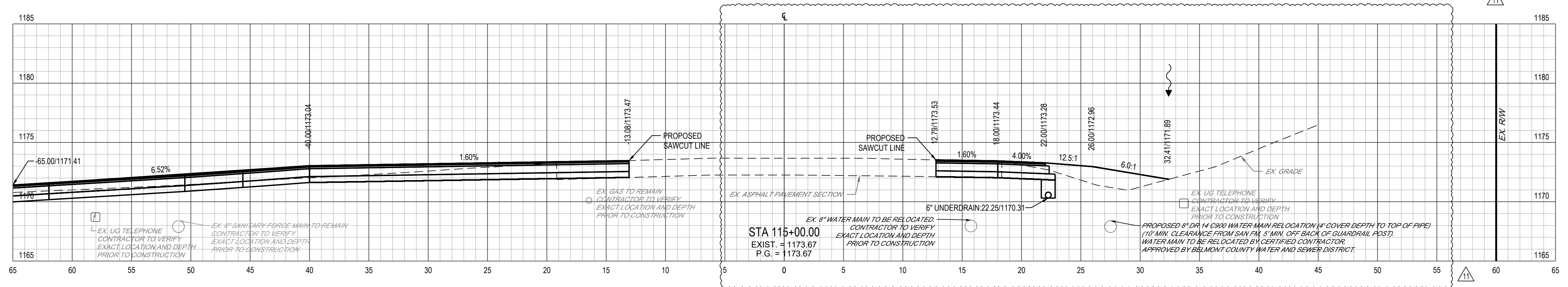
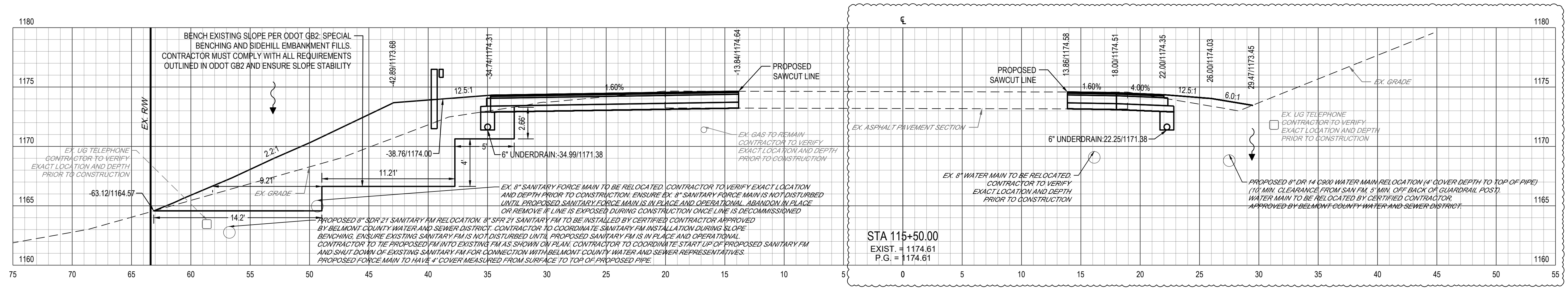
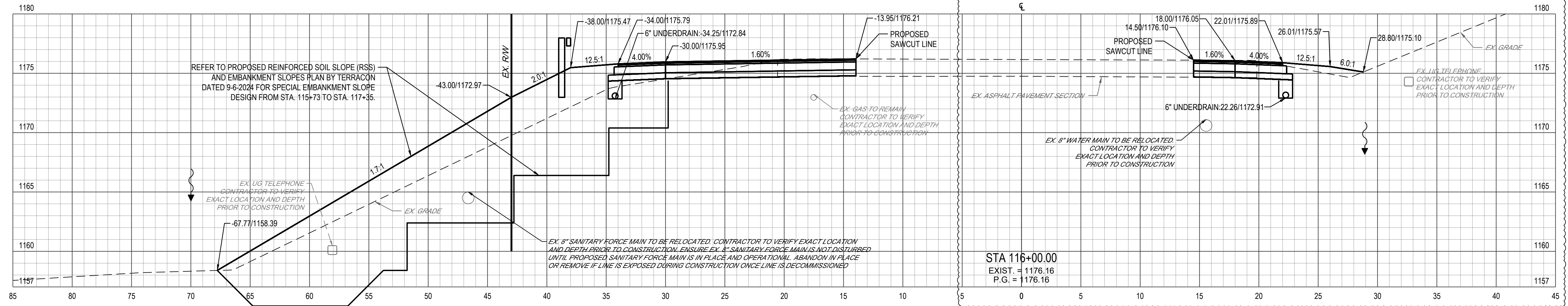
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CROSS SECTIONS - STA. 113+00 TO STA. 114+00
 S.R. 149

BEL-149-23.44

R20
 R32

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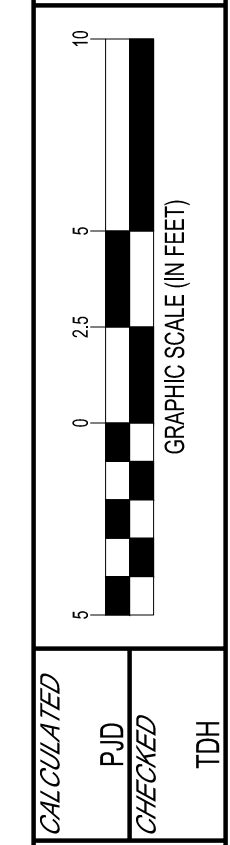
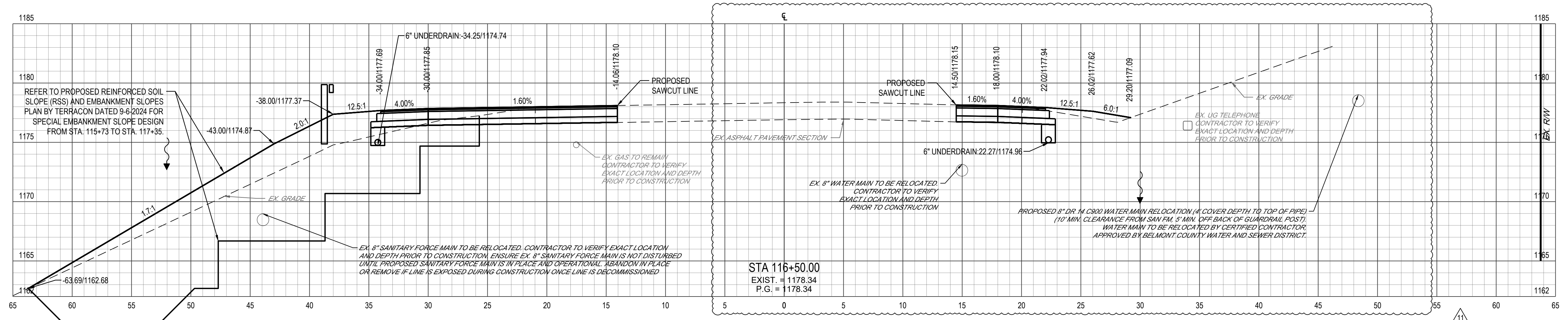
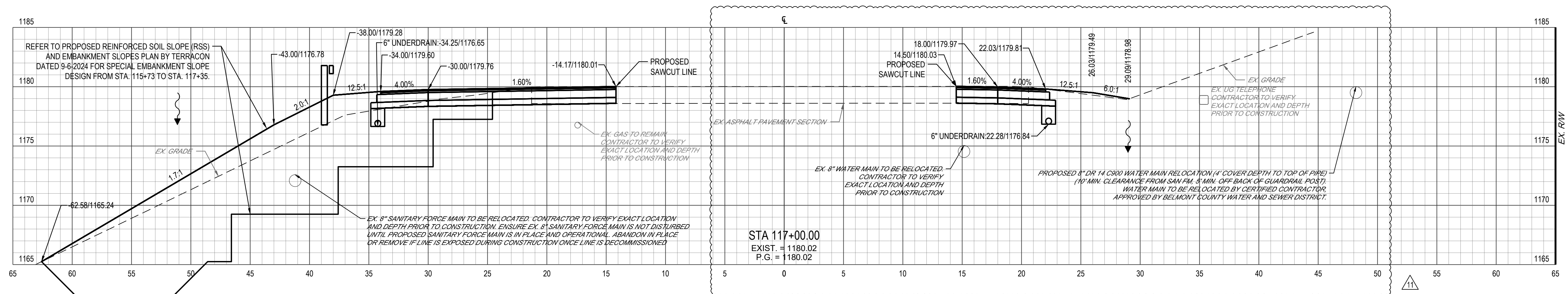
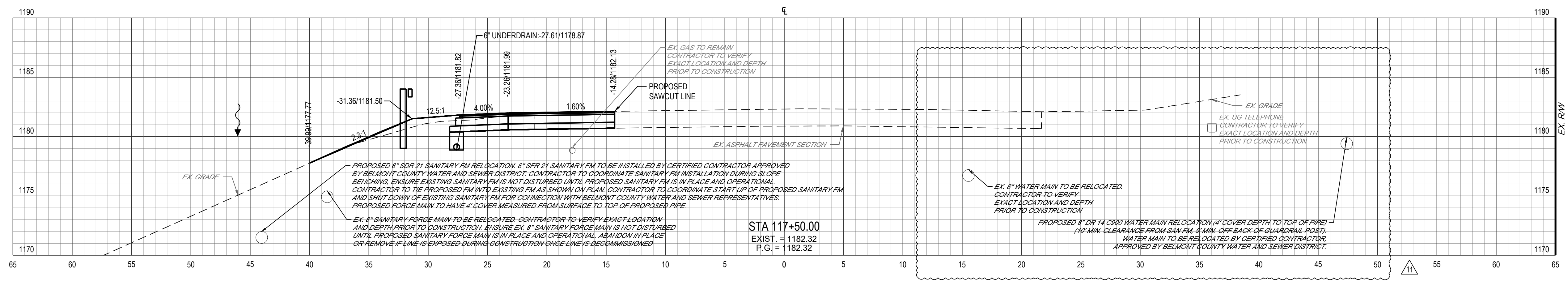
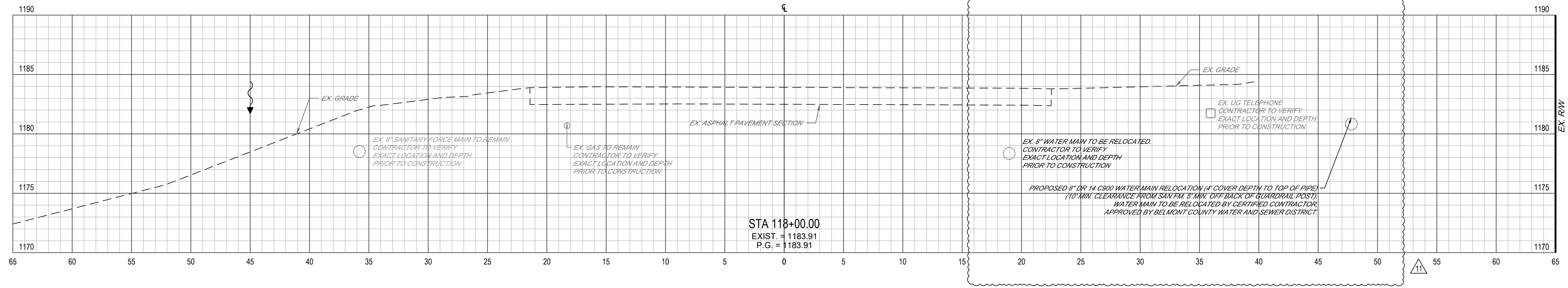


CROSS SECTIONS - STA. 114+50 TO STA. 116+00
S.R. 149

BEL-149-23.44

R21
R32

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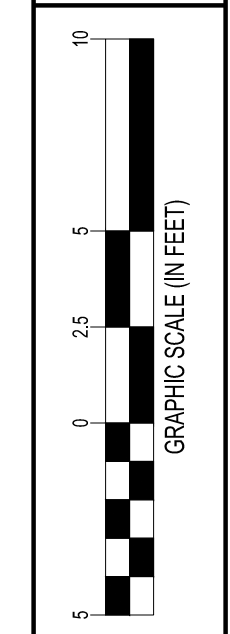
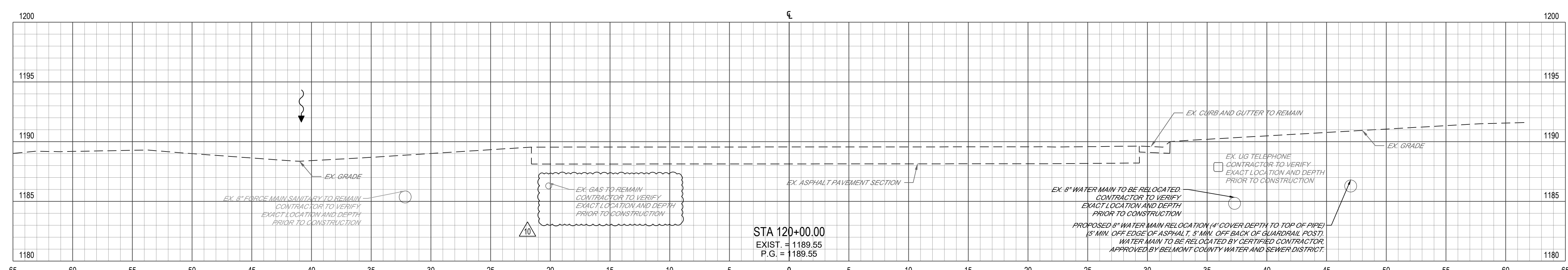
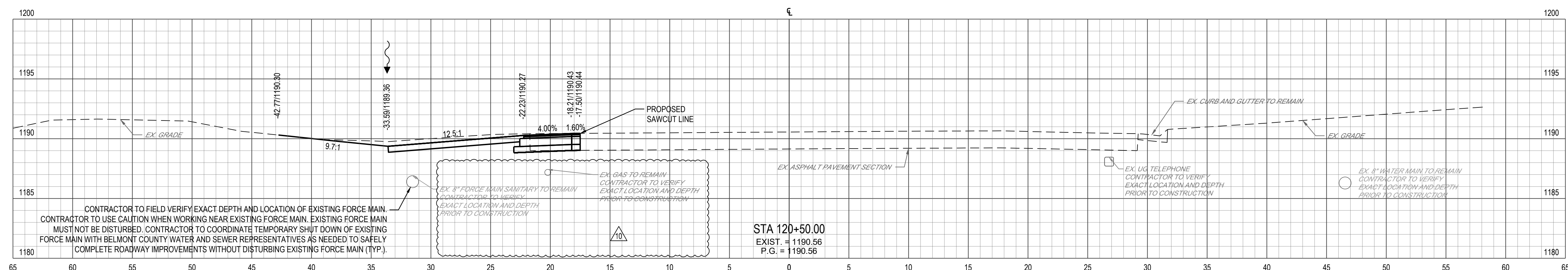
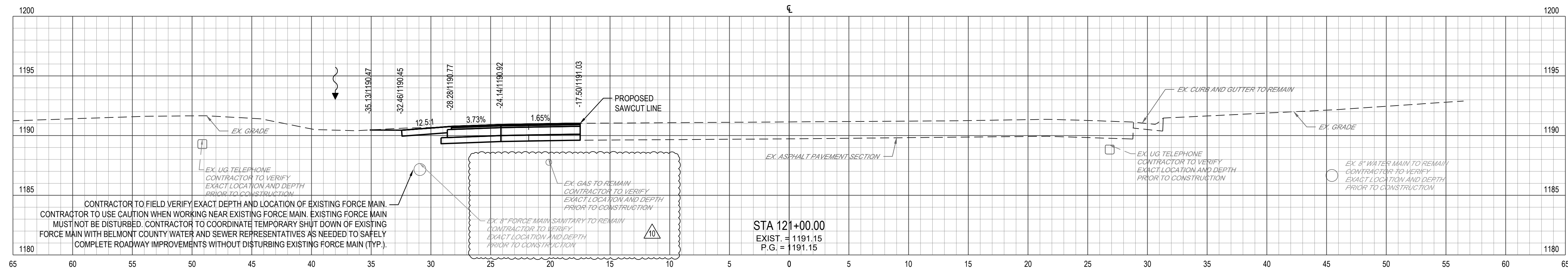
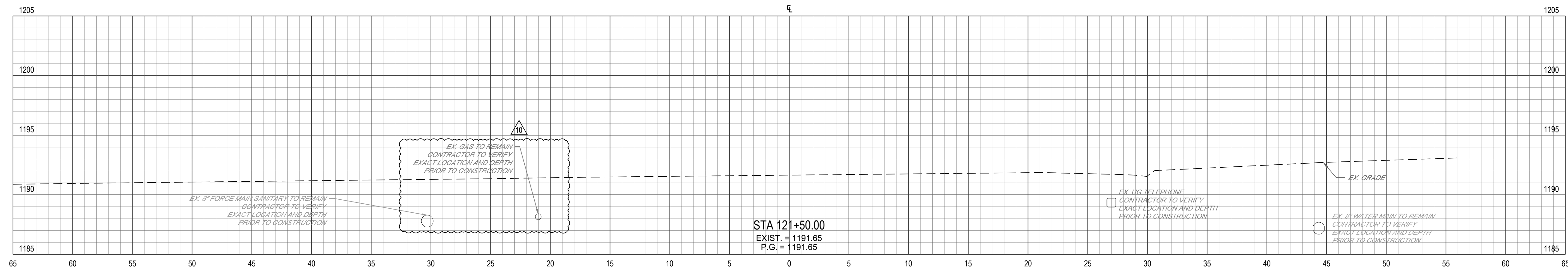


CROSS SECTIONS - STA. 116+50 TO STA. 118+00
 S.R. 149

BEL-149-23.44

R22
 R32

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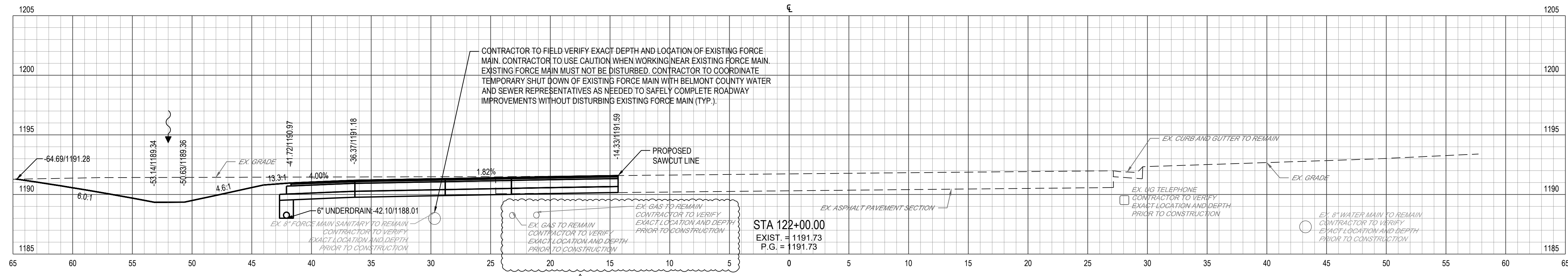
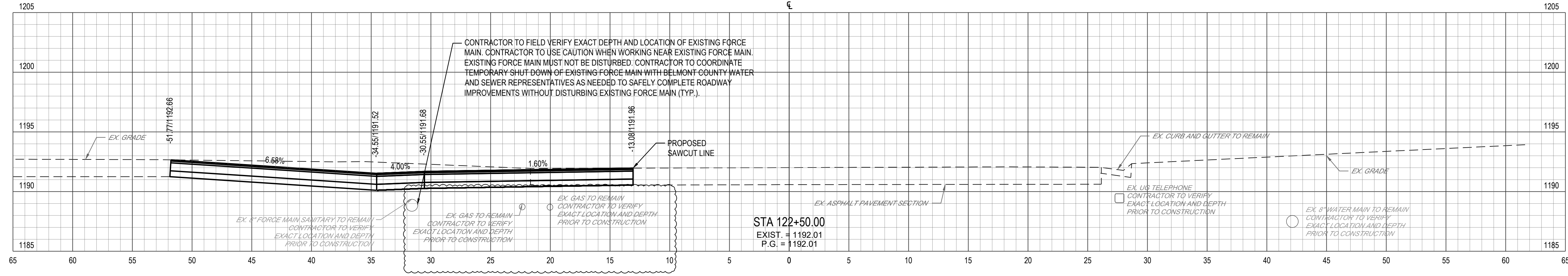
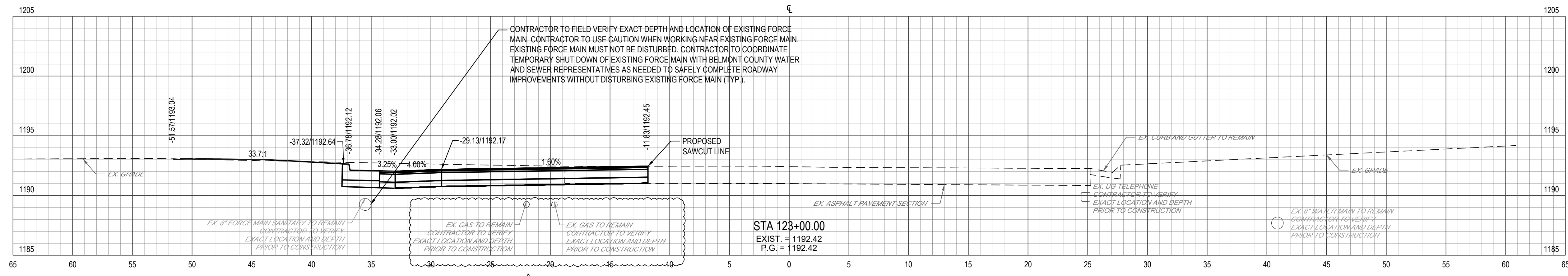
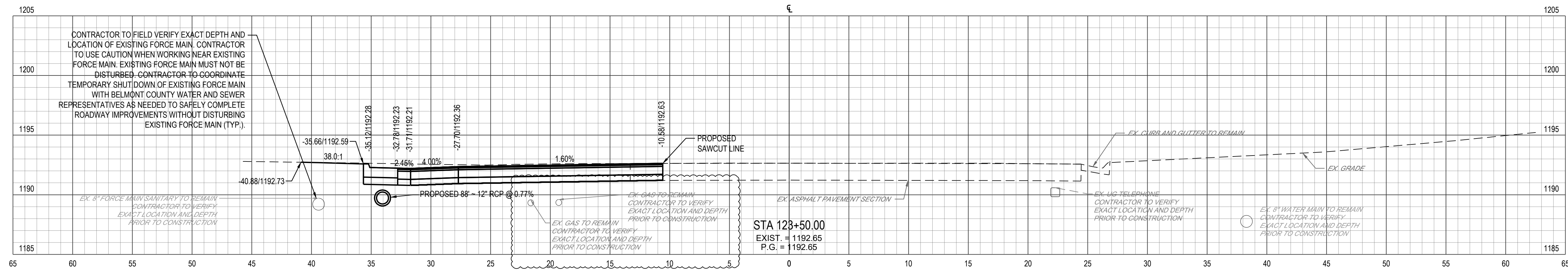


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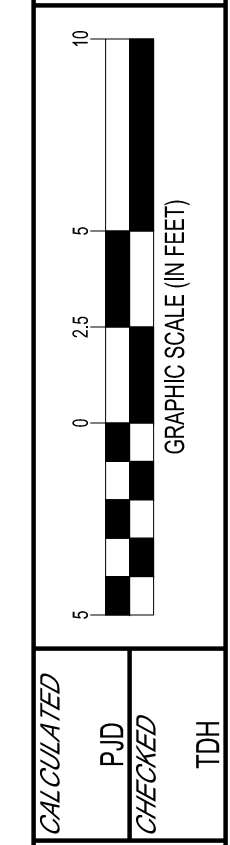
CROSS SECTIONS - STA. 120+00 TO STA. 121+50
S.R. 149

BEL-149-23.44

R23
R32



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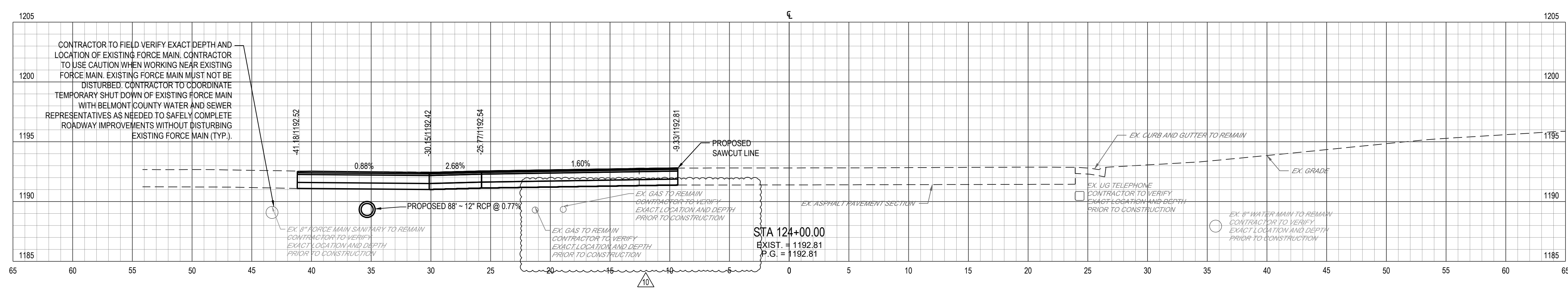
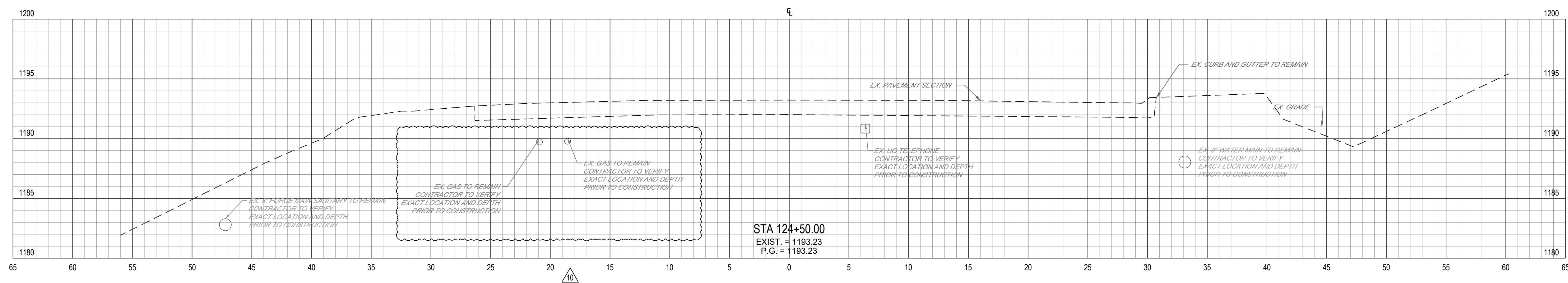
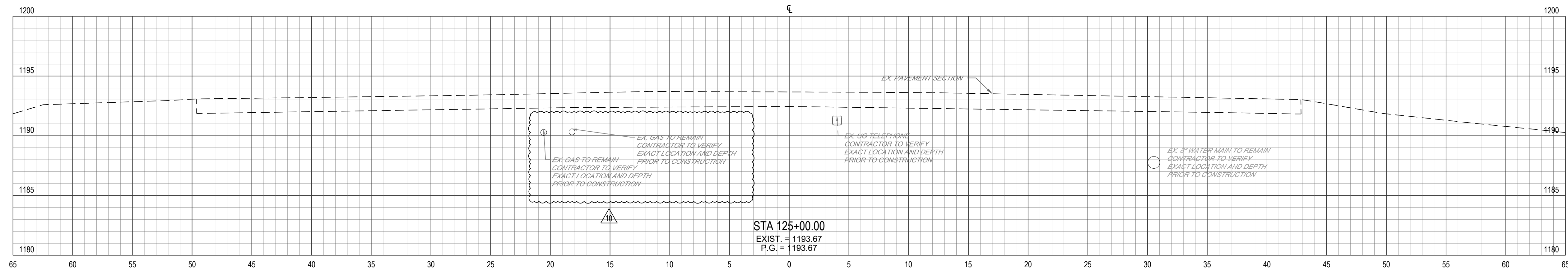
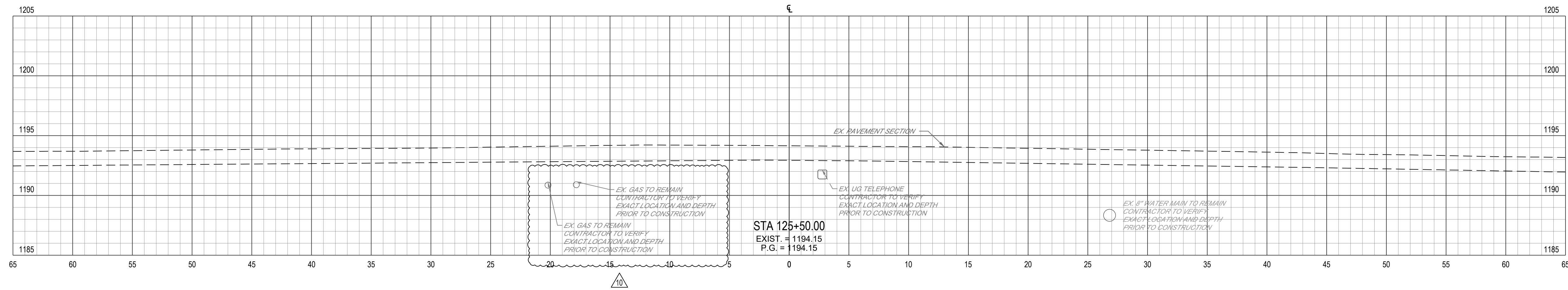


CROSS SECTIONS - STA. 122+00 TO STA. 123+50
S.R. 149

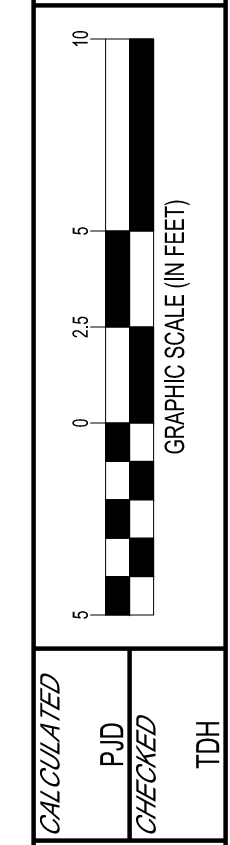
BEL-149-23.44

R24
R32

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CONTRACTOR TO FIELD VERIFY EXACT DEPTH AND LOCATION OF EXISTING FORCE MAIN. CONTRACTOR TO USE CAUTION WHEN WORKING NEAR EXISTING FORCE MAIN. EXISTING FORCE MAIN MUST NOT BE DISTURBED. CONTRACTOR TO COORDINATE TEMPORARY SHUT DOWN OF EXISTING FORCE MAIN WITH BELMONT COUNTY WATER AND SEWER REPRESENTATIVES AS NEEDED TO SAFELY COMPLETE ROADWAY IMPROVEMENTS WITHOUT DISTURBING EXISTING FORCE MAIN (TYP.).

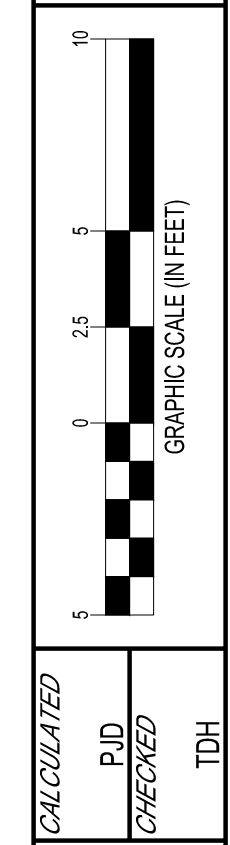
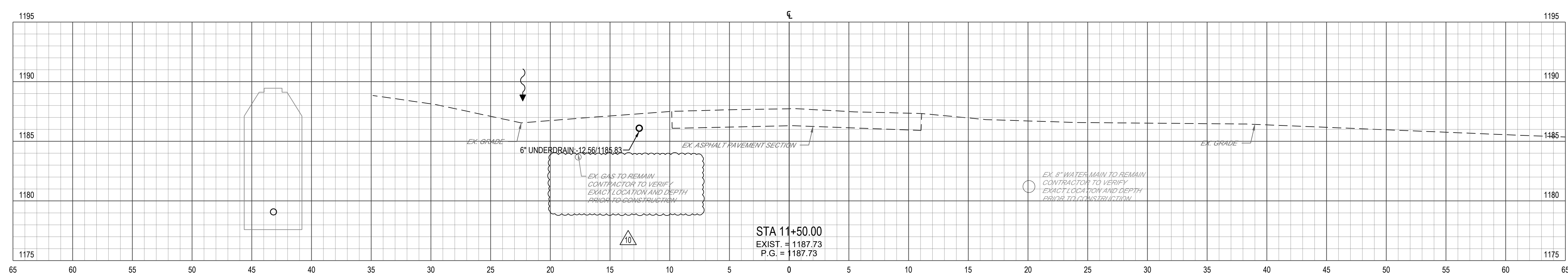
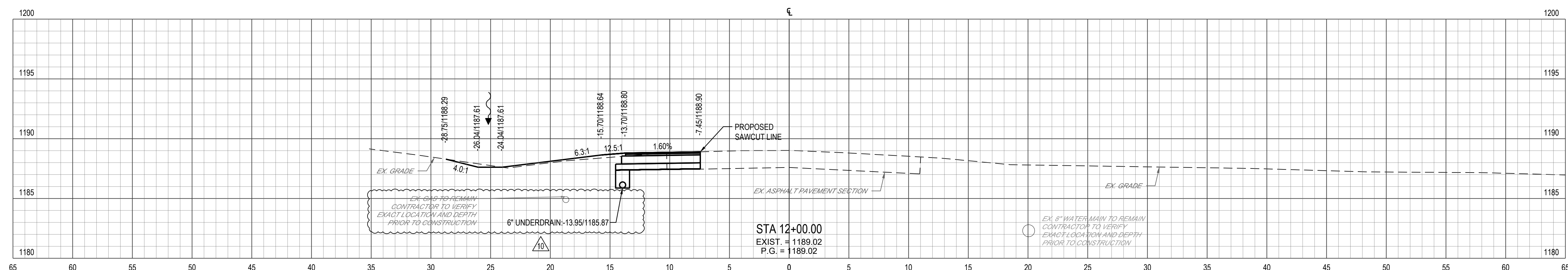
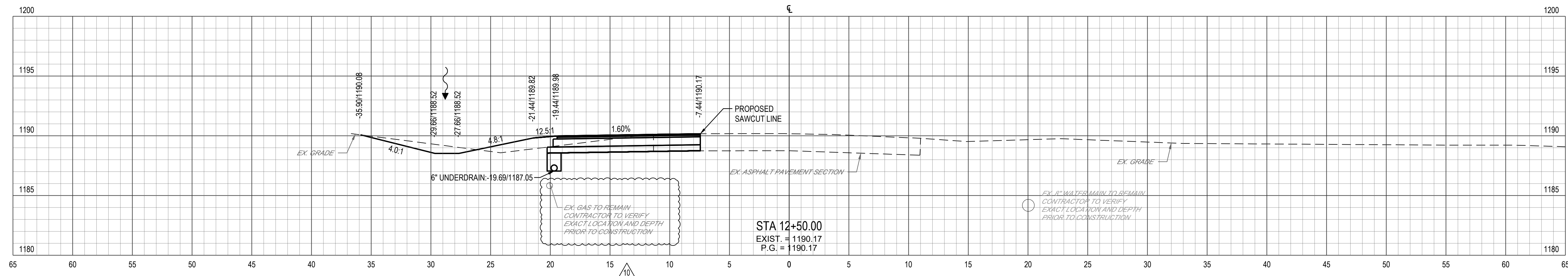
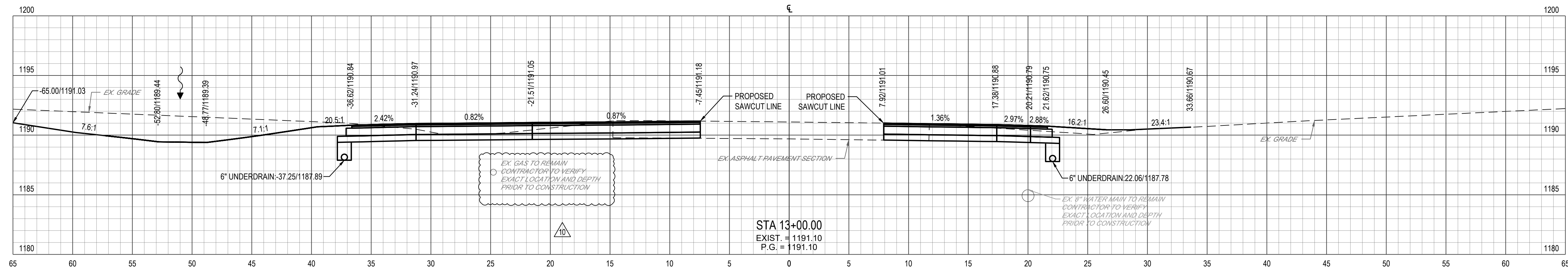


CROSS SECTIONS - STA. 124+00 TO STA. 125+50
S.R. 149

BEL-149-23.44

R25
R32

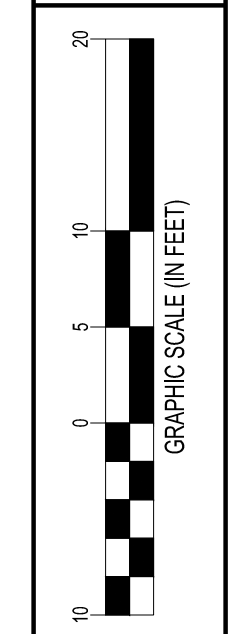
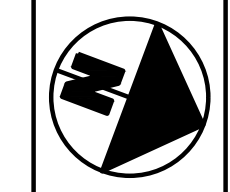
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CROSS SECTIONS - STA. 11+50 TO STA. 13+00
RECO DRIVE

BEL-149-23.44

R26
R32

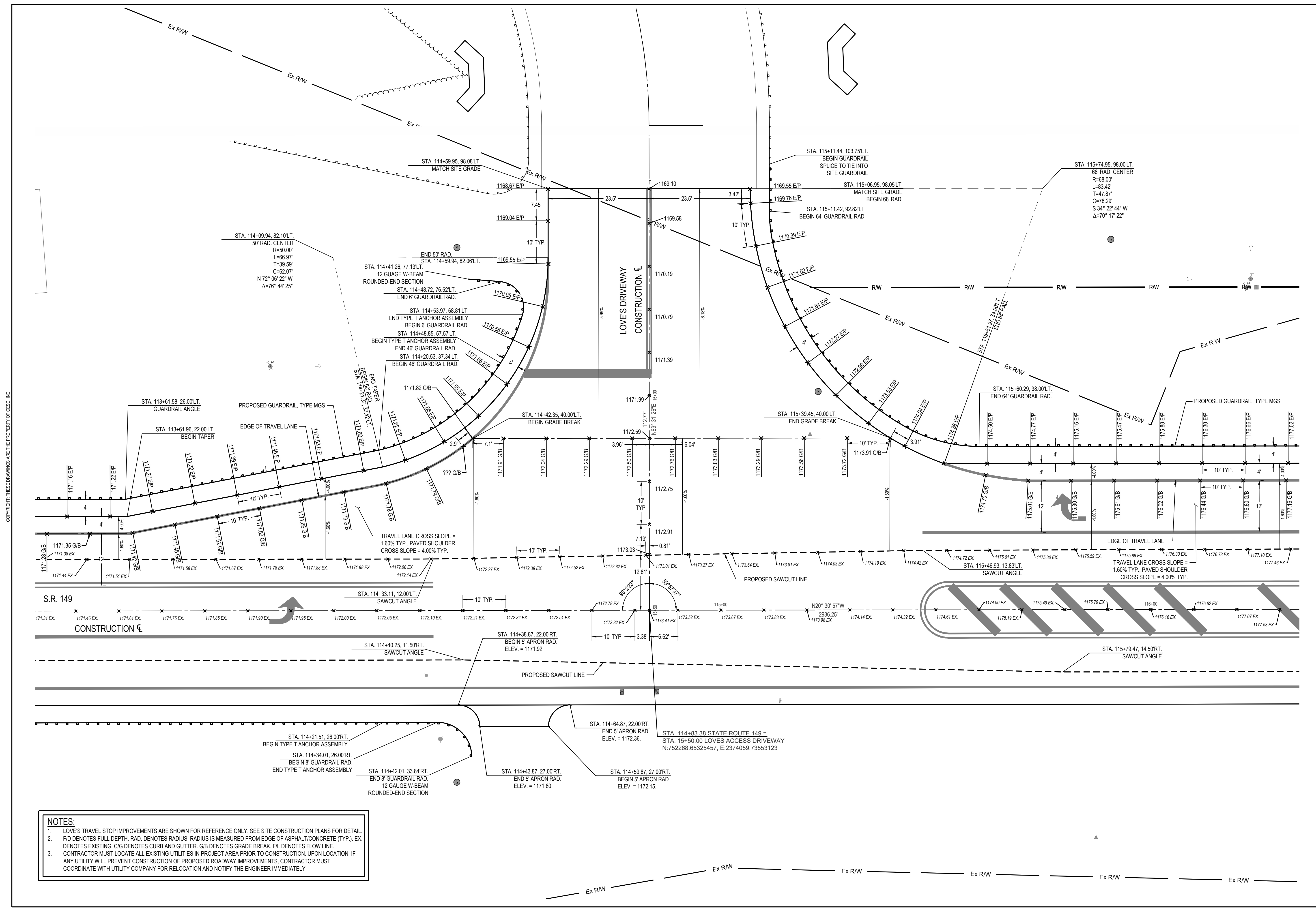


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INTERSECTION DETAIL
S.R. 149

BEL-149-23.44

R27
R32

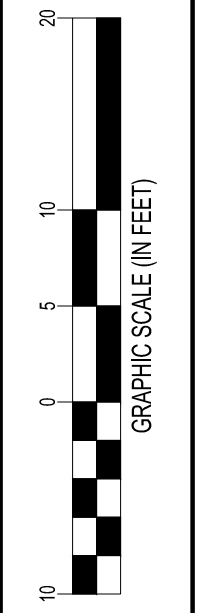
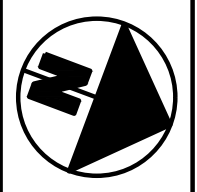


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- NOTES:**
1. LOVE'S TRAVEL STOP IMPROVEMENTS ARE SHOWN FOR REFERENCE ONLY. SEE SITE CONSTRUCTION PLANS FOR DETAIL.
 2. F/D DENOTES FULL DEPTH, RAD. DENOTES RADIUS. RADIUS IS MEASURED FROM EDGE OF ASPHALT/CONCRETE (TYP.). EX. DENOTES EXISTING. C/G DENOTES CURB AND GUTTER. G/B DENOTES GRADE BREAK. F/L DENOTES FLOW LINE.
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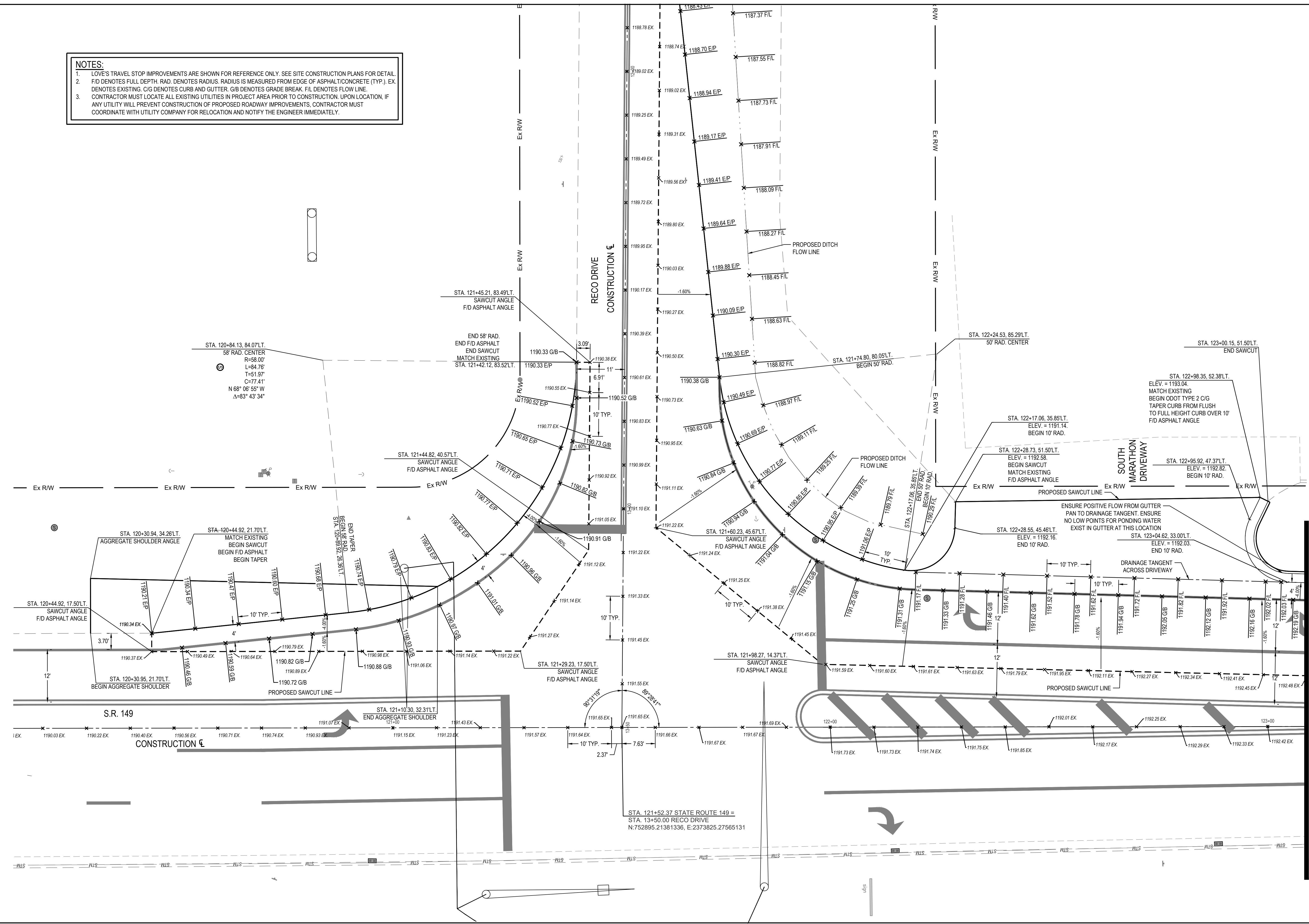
INTERSECTION DETAIL
S.R. 149

MATCHLINE STA 123+10 - SEE SHEET R29

BEL-149-23.44

R28
R32

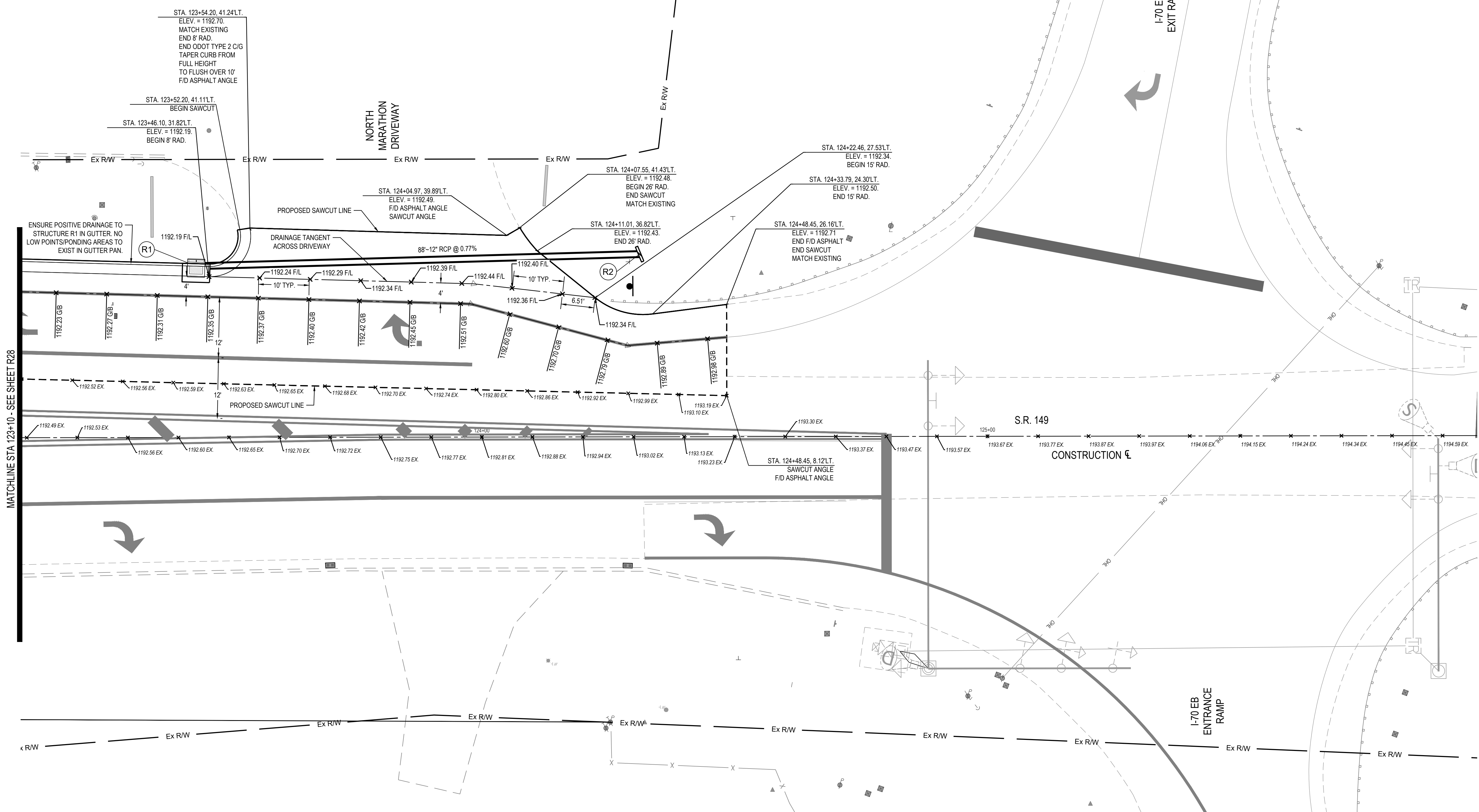
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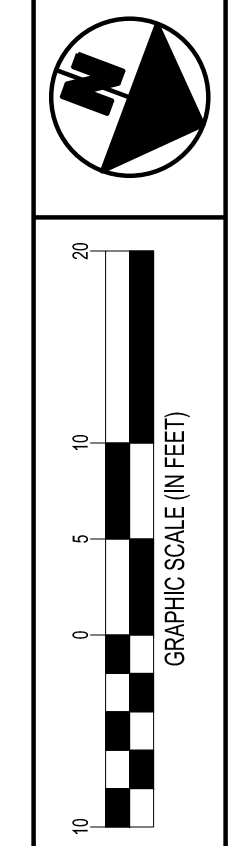
NOTES:

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STORM SEWER STRUCTURE SCHEDULE				
NO.	STRUCTURE	GRATE	INVERT	STATION/OFFSET
R1	CATCH BASIN NO.3A PER ODOT SCD CB-3A	1192.01	1189.28 (12") N	STA. 123+43.57, 33.89'LT.
R2	HALF-HEIGHT HEADWALL PER ODOT SCD HW-2.2		1188.60 (12") S	STA. 124+31.25, 36.15'LT.



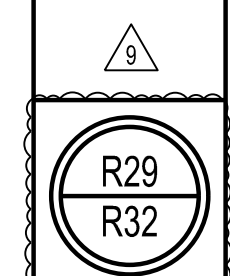
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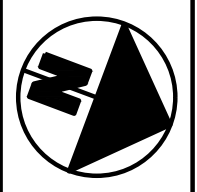
INTERSECTION DETAIL
S.R. 149

BEL-149-23.44

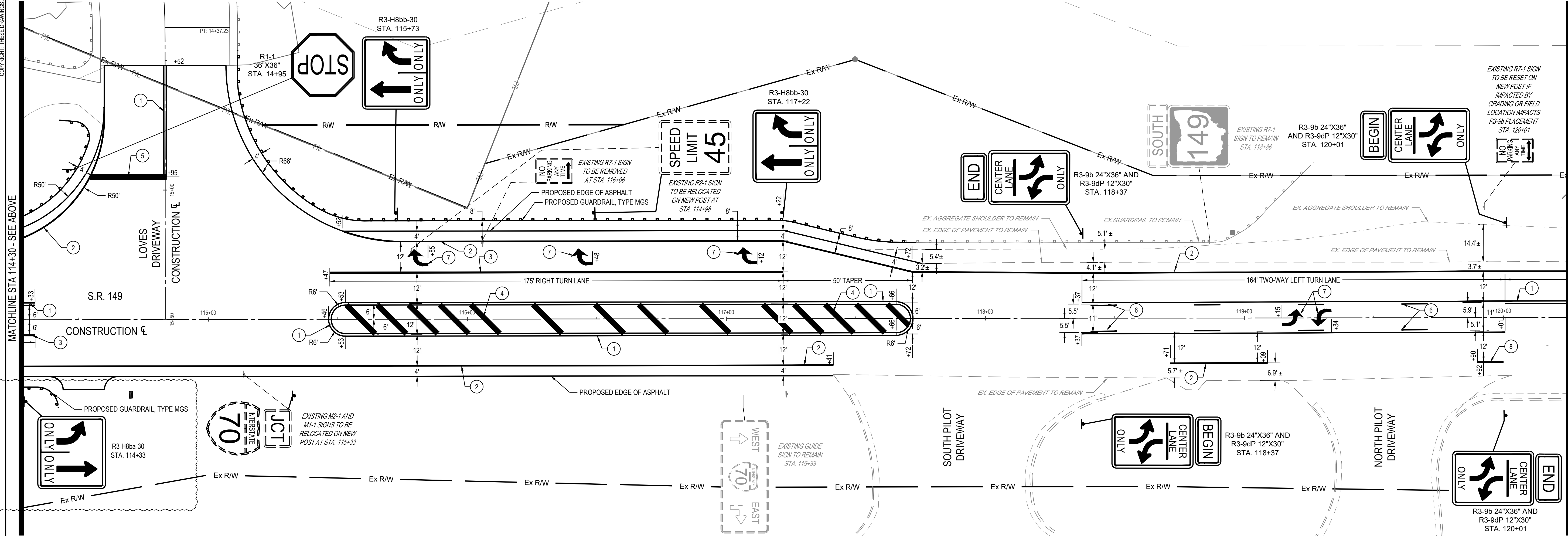
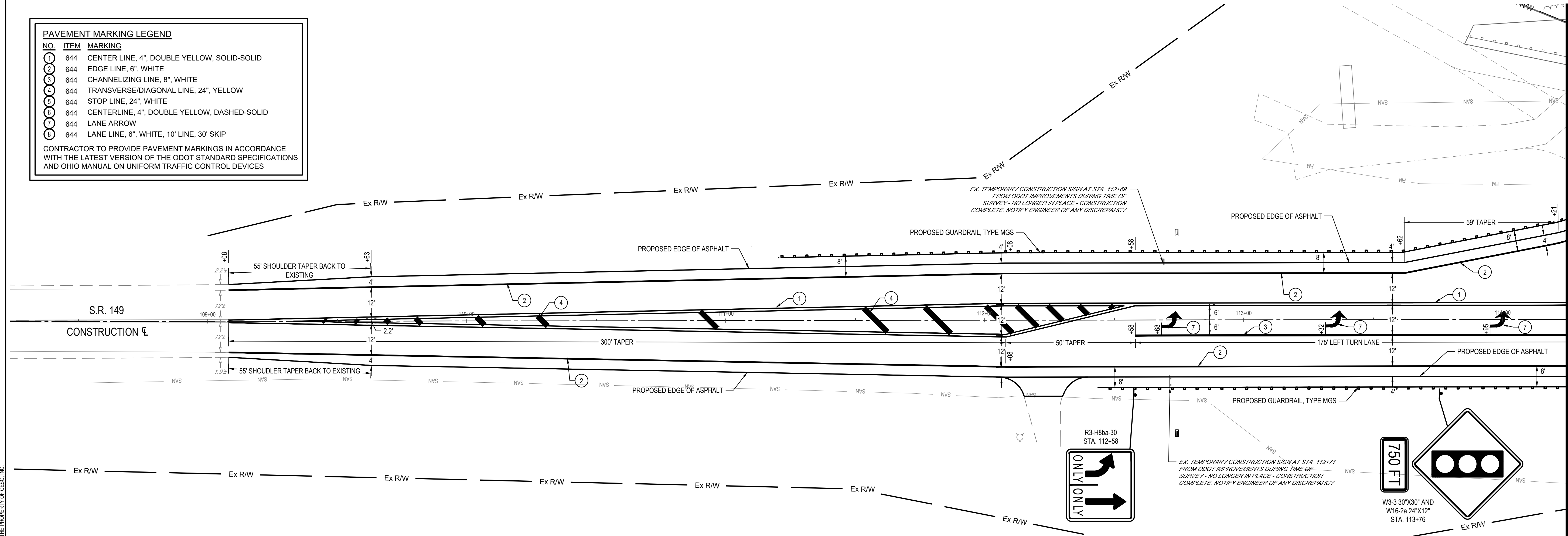


NO.	ITEM	MARKING
1	644	CENTER LINE, 4", DOUBLE YELLOW, SOLID-SOLID
2	644	EDGE LINE, 6", WHITE
3	644	CHANNELIZING LINE, 8", WHITE
4	644	TRANSVERSE/DIAGONAL LINE, 24", YELLOW
5	644	STOP LINE, 24", WHITE
6	644	CENTERLINE, 4", DOUBLE YELLOW, DASHED-SOLID
7	644	LANE ARROW
8	644	LANE LINE, 6", WHITE, 10' LINE, 30' SKIP

CONTRACTOR TO PROVIDE PAVEMENT MARKINGS IN ACCORDANCE WITH THE LATEST VERSION OF THE ODOT STANDARD SPECIFICATIONS AND OHIO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES



SCALE IN FEET
 P.D. CHECKED
 TDH



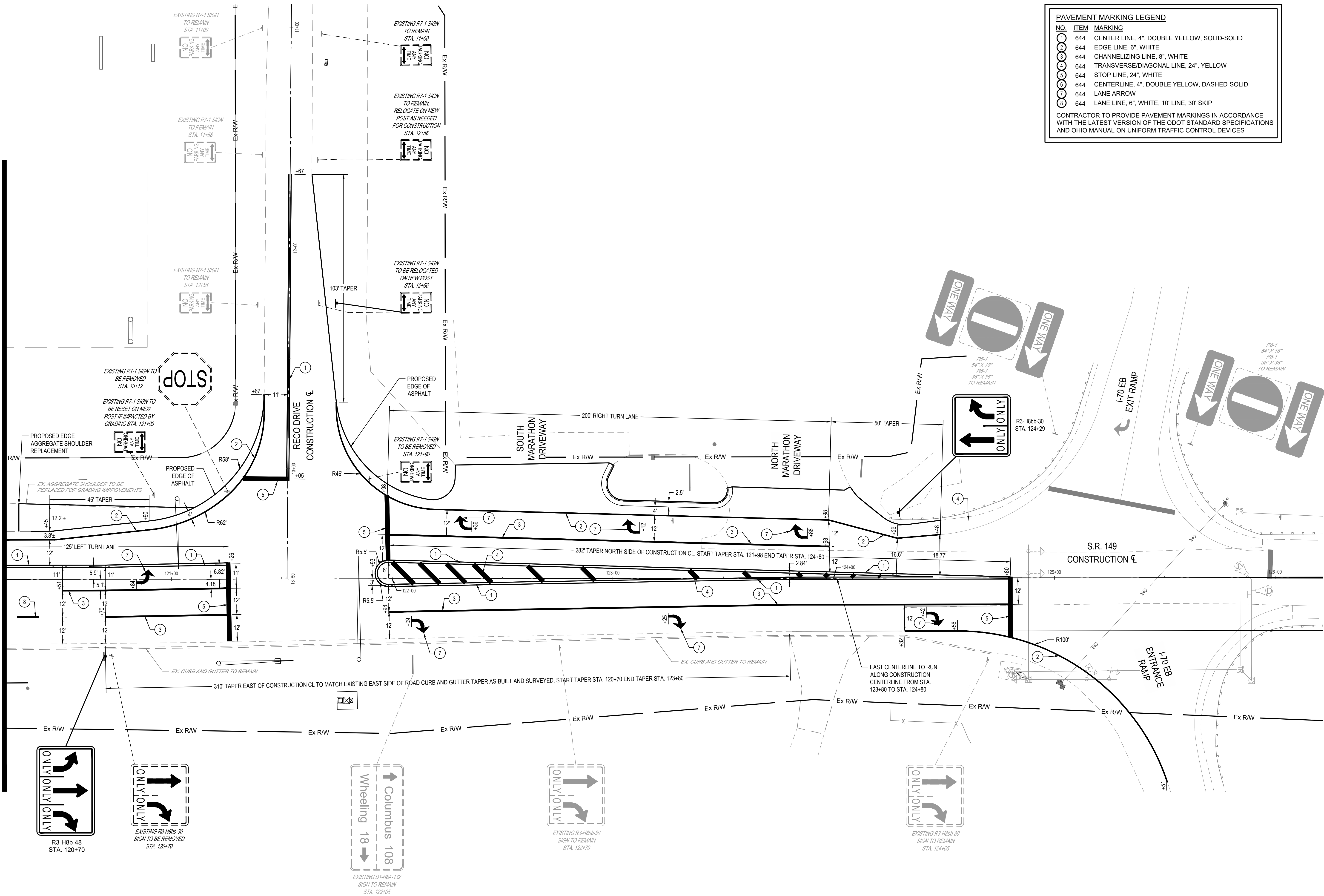
PAVEMENT MARKING & SIGNAGE - STA. 108+50 TO STA. 120+25

BEL-149-23.44



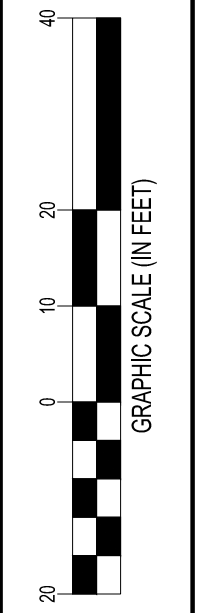
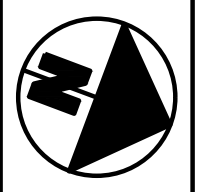
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MATCHLINE STA 120+25 - SEE SHEET R30



PAVEMENT MARKING LEGEND		
1	644	CENTER LINE, 4", DOUBLE YELLOW, SOLID-SOLID
2	644	EDGE LINE, 6", WHITE
3	644	CHANNELIZING LINE, 8", WHITE
4	644	TRANSVERSE/DIAGONAL LINE, 24", YELLOW
5	644	STOP LINE, 24", WHITE
6	644	CENTERLINE, 4", DOUBLE YELLOW, DASHED-SOLID
7	644	LANE ARROW
8	644	LANE LINE, 6", WHITE, 10' LINE, 30' SKIP

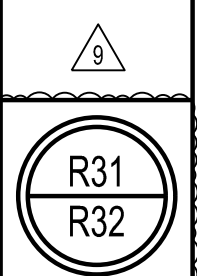
CONTRACTOR TO PROVIDE PAVEMENT MARKINGS IN ACCORDANCE WITH THE LATEST VERSION OF THE ODOT STANDARD SPECIFICATIONS AND OHIO MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES



CALCULATED
P.I.D.
CHECKED
TDH

PAVEMENT MARKING & SIGNAGE - STA. 120+25 TO 126+00
S.R. 149

BEL-149-23.44



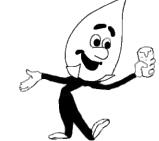
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GENERAL CONSTRUCTION REQUIREMENTS

1. DETAIL PLANS OF PLANNED WATER LINE CONSTRUCTION WILL BE SUBMITTED TO THE BELMONT COUNTY WATER AND SEWER DISTRICT FOR APPROVAL BY THE ENGINEER ASSOCIATE, PRIOR TO CONSTRUCTION. A PRECONSTRUCTION MEETING WILL BE REQUIRED BEFORE START OF CONST.
2. SIGNED WATER APPLICATIONS & APPLICABLE FEES MUST BE RECEIVED PRIOR TO CONSTRUCTION.
3. ALL WATERLINES SHALL BE C900 (DR-14) IN HIGH PRESSURE AREAS DUCTILE IRON CLASS 50 OR 52 WILL BE REQUIRED BY THE DISTRICT. (ALL DUCTILE IRON PIPE TO BE POLYETHYLENE WRAPPED AS PER AWWA C105/A21.5) PIPE SIZES TO BE 3/4"CTS, 1"CTS, 2" P., 4", 6", 8", 10", 12" C-900 DR 14-18 (3/4" OR 1" TO BE TYPE-K SOFT COPPER IF LINE PRESSURE IS GREATER THAN 200 PSI)
4. RESIDENTIAL PVC SEWER LINE WILL BE A MINIMUM OF 4 INCH SDR-35.
5. SEWER FORCE MAIN WILL BE A MINIMUM OF 1 1/2", OR 2" POLYETHYLENE CLASS 200, OR 2", 4", 6", OR 8" SDR-21 CL-200 PVC PIPE, AND GREEN IN COLOR. GRAVITY SEWER LINES SHALL BE AIR AND DEFLECTION TESTED IN ACCORDANCE WITH TABLE 1 PAGE 29 OF THIS BOOKLET.
6. ALL PIPE THAT IS INSTALLED SHALL BE BEDDED WITH A MINIMUM OF 6" ABOVE AND BELOW THE PIPE WITH GOOD TOP SOIL FREE OF STONE AND OTHER FOREIGN OBJECTS, OR CRUSH AND RUN STONE.
7. WATERLINE SHALL BE INSTALLED WITH 4 FOOT MINIMUM COVER UNLESS OTHERWISE APPROVED.
8. A MINIMUM VERTICAL CLEARANCE OF 18 INCHES SHALL BE PROVIDED BETWEEN THE PROPOSED WATER MAIN AND OTHER UTILITIES OR STRUCTURES, UNLESS OTHERWISE NOTED.
9. A MINIMUM HORIZONTAL CLEARANCE OF 4 FEET SHALL BE PROVIDED BETWEEN THE PROPOSED WATER LINE AND OTHER UTILITIES OR STRUCTURES UNLESS OTHERWISE APPROVED.
10. THE WATERLINE SHALL BE PLACED WITH A MINIMUM OF 10 FOOT HORIZONTAL SEPARATION FROM ANY SANITARY SEWERS OR WHEN THE WATER LINE CROSSES A SANITARY SEWER, THE WATER LINE SHALL BE A MINIMUM OF 18 INCHES ABOVE THE SEWER.
11. SEWER LINES MUST CROSS UNDER ALL WATERLINES WITH A MINIMUM OF 18" OF CLEARANCE. IF NOT POSSIBLE THE BCSD WILL PERMIT CROSSING OVER WITH STEEL CASING OR PLASTIC CASING AND CONCRETE. MINIMUM CASING LENGTH IS 10FT. AS PER NOTE ON SHT. #16
12. ALL FITTINGS SHALL BE MECHANICAL JOINT (M.J), FUSION EPOXY COATED, DUCTILE IRON, RESTRAINED BY THE USE OF RESTRAINERS AND/OR MEGALUGS AND THRUST BLOCKED WITH CONCRETE AS REQUIRED. SEE SHT# 25 FOR THRUST BLOCKING DETAIL.
13. A 16FT. MIN. LENGTH OF WATERLINE SHALL BE PROVIDED ON EACH SIDE OF ANY MAINLINE FITTING, OR THE JOINT SHALL BE RESTRAINED WITH BELL RESTRAINT FITTINGS.
14. ALL SERVICE CONNECTIONS MADE TO EXISTING LINES WILL BE MADE BY THE DISTRICT. THE CONNECTION SHALL BE MADE WET USING A TAPPING TEE AND VALVE. IF A LINE TO BE CONNECTED IS THE SAME AS THE EXISTING LINE, 2" CONNECTED TO 2" OR 6" CONNECTED TO 6". THE CONNECTION WILL BE MADE DRY. EXCEPTIONS MAY BE MADE DUE TO UNFORESEEN FIELD CIRCUMSTANCES.

CONTINUES ON NEXT PAGE

BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



12/14/17


2

GENERAL CONSTRUCTION REQUIREMENTS

15. MAGNETIC DETECTABLE TRACER TAPE SHALL BE INSTALLED AT 18 INCHES DEEP WITH ALL PVC PIPE.
16. TRACE WIRE TO BE INSTALLED ON ALL WATER & SEWER LINES.
 - A.) OPEN-TRENCH INSTALLATION: DIRECT BURIAL #12 AWG SOLID (0.0808" DIA.), STEEL CORE SOFT DRAWN TRACER WIRE, 250# AVERAGE TENSILE BREAK LOAD, 30 MIL HIGH MOLECULAR-HIGH DENSITY POLYETHYLENE JACKET COMPLYING WITH ASTM-D-1248, 30 VOLT RATING, 45 MIL JACKET WHEN DIRECTIONAL BORED.
 - B.) CONNECTORS
SPICES ALONG THE CONTINUOUS RUN OF TRACE WIRE FOR REPAIR OF A WIRE BREAK OR REPLACEMENT OF FAILED SEGMENT OF WIRE SHALL USE 3M BRAND DBR DIRECT BURY SPLICE KIT OR APPROVED EQUAL.
 - C.) INSTALLATION
TRACE WIRE ACCESS POINT SHALL BE ACCESSIBLE AT ALL NEW WATER VALVE BOXES, WATER METER BOXES, BLOWOFFS, ARVs, FIRE HYDRANTS AND MANHOLES.
 - D.) TESTING REQUIREMENTS
CONTRACTOR SHALL PERFORM A CONTINUITY TEST ON ALL TRACE WIRE IN THE PRESENCE OF BCSD. IF THE TRACE WIRE IS FOUND TO BE NOT CONTINUOUS AFTER TESTING, CONTRACTOR SHALL REPAIR OR REPLACE THE FAILED SEGMENT OF WIRE.
17. THE CONTRACTOR WILL SCHEDULE WITH THE DISTRICT APPROXIMATE TIMES FOR THE INSPECTION OF LINES, FITTINGS & ETC., PRIOR TO BACKFILLING. THE COUNTY SHALL BE GIVEN A MINIMUM 48 HOURS OF NOTICE WHEN AN INSPECTION IS REQUIRED.
18. EXISTING MAINLINE VALVES WILL ONLY BE OPERATED BY DISTRICT PERSONNEL.
19. A MINIMUM OF 24 HOURS NOTICE FOR ANY INSTALLATIONS BY CONTRACTORS THAT MAY DISRUPT WATER SERVICE TO EXISTING WATER CUSTOMERS. THE NOTICE SHALL BE MADE THROUGH THE DISTRICTS OFFICE.
20. ALL SERVICE CONNECTIONS INSTALLED WILL BE STAKED AT THE TIME OF CONSTRUCTION WITH IRON "TEE BAR STAKES" PER DISTRICT REQUIREMENTS.
21. CHANGES IN GRADE AND ALIGNMENT TO BE OBTAINED BY DEFLECTIONS AT PIPE JOINTS SHALL BE NO GREATER THAN RECOMMENDED BY THE MANUFACTURER OF THE WATER LINE.
22. PRESSURE TESTING OF WATER LINES AND SEWER FORCE MAINS SHALL BE IN ACCORDANCE WITH AWWA C600-87. TESTING AT 1-1/2 TIMES EXISTING PRESSURE AND/OR WORKING PRESSURE OR MINIMUM OF 100psi/MAX. OF 200 psi. PRESSURE TESTING MUST BE DONE WITHIN 2 WORKING WEEKS ONCE LINE IS IN PLACE.
23. THE CONTRACTOR WILL CHLORINATE THE INSTALLED WATER LINE AT RECOMMENDED AWWA 651 STANDARDS, FOR A MINIMUM OF 24 HOURS, AT 50 P.P.M.
24. THE DISTRICT WILL TAKE REPRESENTATIVE SAMPLES OF THE WATER IN THE LINES, AND HAVE AN APPROVED LABORATORY MAKE THE BACTERIOLOGICAL EXAMINATIONS OF THE SAMPLE TO ESTABLISH THE ACCEPTABILITY OF THAT PORTION OF THE SYSTEM PRIOR TO ANY TURN ON OF SERVICE TO ANY FACILITY. THERE WILL BE NO EXCEPTION TO THIS STANDARD.

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BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



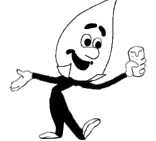
10/31/17

3

GENERAL CONSTRUCTION REQUIREMENTS

25. ALL MAIN LINE VALVES & FLUSH-OUT ASSEMBLIES SHALL BE MARKED BY A WHITE FIBERGLASS MARKER AS PER THE WATER DISTRICTS REQUIREMENTS.
26. LOCATION OF THE PROPOSED SERVICE TAPS SHALL BE FIELD LOCATED BY THE OWNER PRIOR TO CONSTRUCTION OR AT TIME OF CONSTRUCTION.
27. ALL MATERIAL AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE BELMONT COUNTY SPECIFICATIONS AS OUTLINED IN STANDARD DETAILS MANUAL.
28. ALL PARTS, LABOR, EQUIPMENT, AND MISC. EXPENSES BY THE BELMONT COUNTY SANITARY SEWER DISTRICT WILL BE INVOICED TO THE DEVELOPER.
29. CONTRACTOR IS RESPONSIBLE FOR ALL WORK PERFORMED FOR THE FIRST YEAR OF CUSTOMER SERVICE.
30. ALL PRESSURE TESTING STARTS BEFORE NOON.
31. AS-BUILTS DRAWINGS (MAPS) MUST BE PROVIDED TO BCSD WITHIN 3 MONTHS OF THE BACTERIA TEST, WHICH MARKS THE WATER LINE COMPLETION DATE. FOR 6" AND LARGER LINES/ AND WITHIN ONE MONTH FOR 2" & 4" LINES.
32. AS-BUILTS FOR 6" & LARGER LINES ARE REQUIRED TO HAVE GPS POINT WHEN SUBMITTED FOR APPROVAL THEY ARE TO POINT OUT THE FOLLOWING: HYDRANTS, FITTINGS, CURB BOXES, METER WELLS, MAN HOLES, CLEAN OUTS, VALVES, AND WATER OR SEWER LINES IN 100FT. INCREMENTS.

BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO

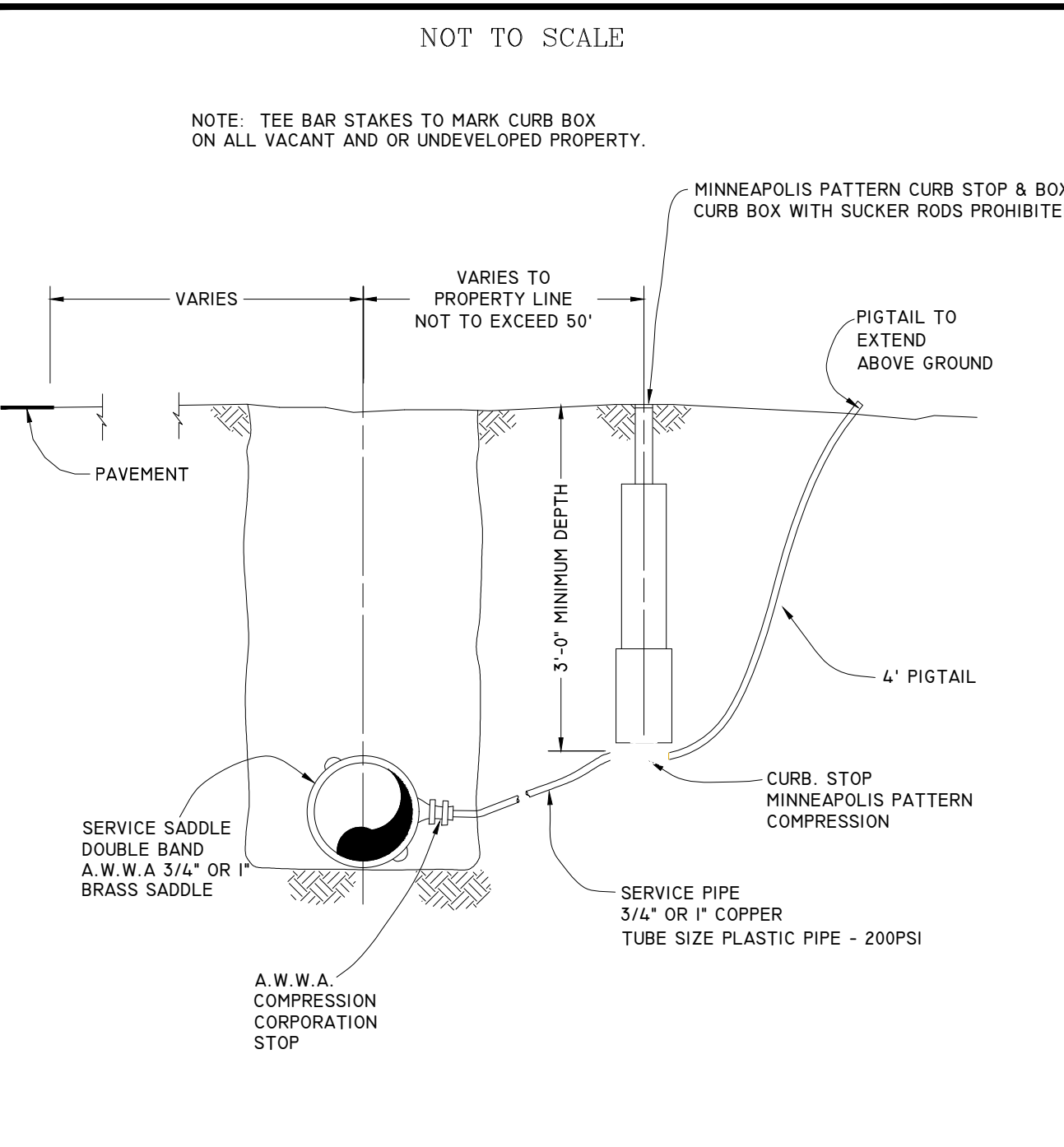


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3A

NOT TO SCALE

NOTE: TEE BAR STAKES TO MARK CURB BOX ON ALL VACANT AND OR UNDEVELOPED PROPERTY.



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



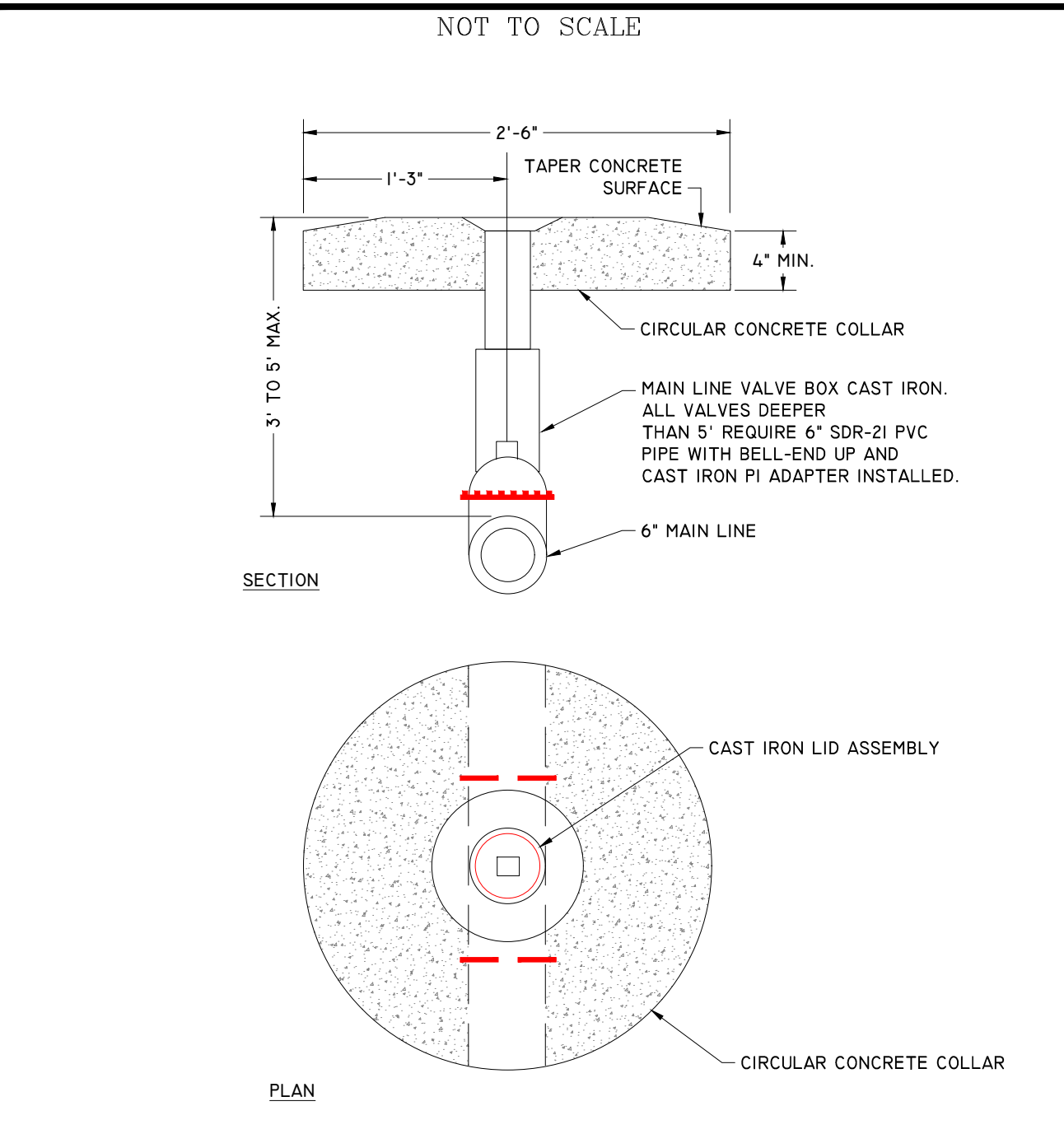
STANDARD DETAILS
INSIDE SERVICE SETTING

DWG. NO.
2

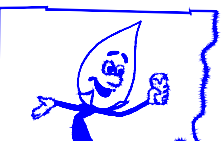
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5

NOT TO SCALE



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



STANDARD DETAILS
VALVE BOX SETTING

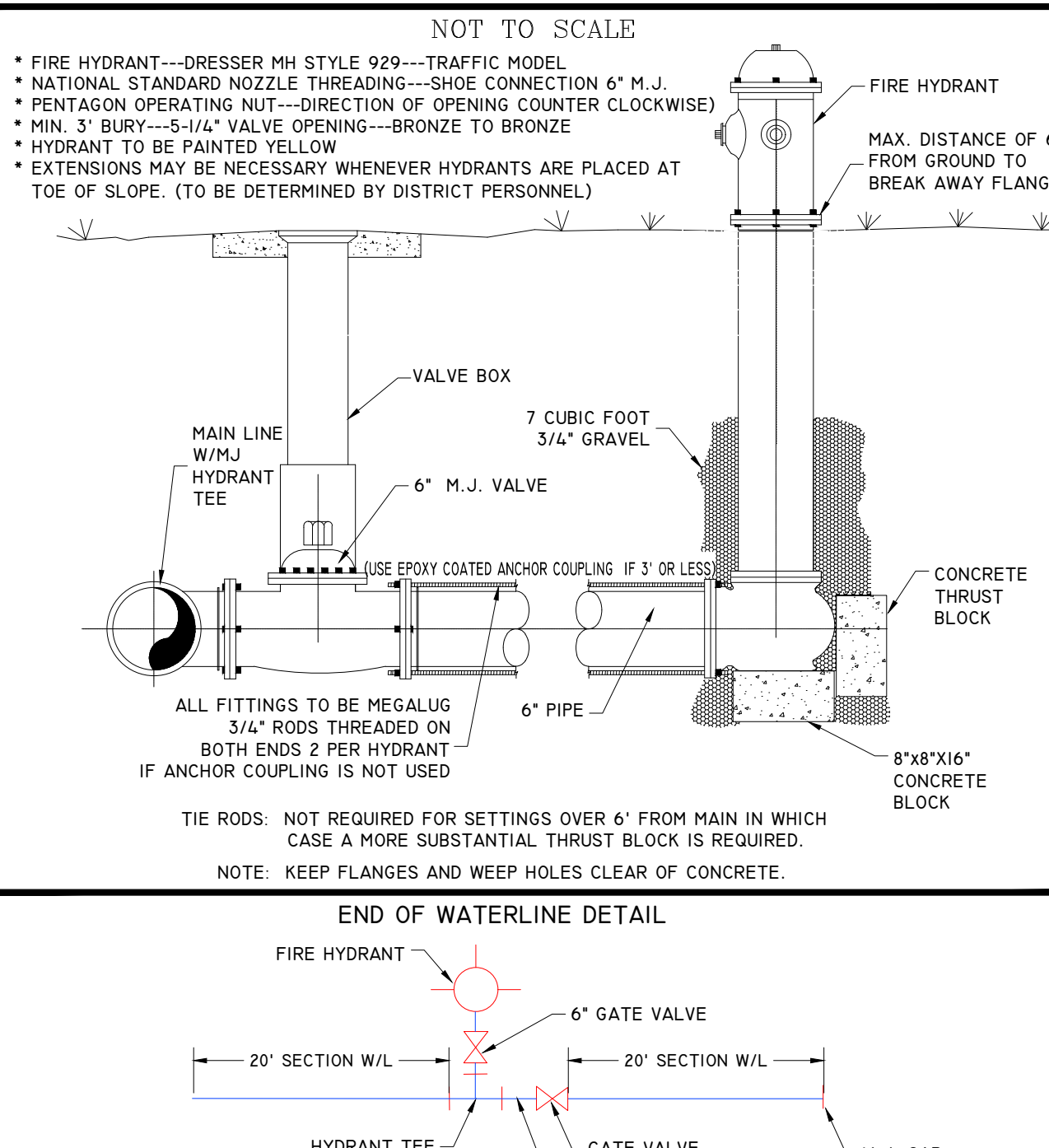
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10/31/17

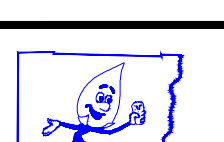
10

NOT TO SCALE

- FIRE HYDRANT---DRESSER MH STYLE 929---TRAFFIC MODEL
- NATIONAL STANDARD NOZZLE THREADING---SHOE CONNECTION 6" M.J.
- PENTAGON OPERATING NUT---DIRECTION OF OPENING COUNTER CLOCKWISE
- MIN. 3' BURY---5-1/4" VALVE OPENING---BRONZE TO BRONZE
- HYDRANT TO BE PAINTED YELLOW
- EXTENSIONS MAY BE NECESSARY WHENEVER HYDRANTS ARE PLACED AT TOE OF SLOPE. (TO BE DETERMINED BY DISTRICT PERSONNEL)



BELMONT COUNTY WATER AND SEWER DISTRICT
BELMONT COUNTY, OHIO



STANDARD DETAILS
FIRE HYDRANT ASSEMBLY
ARRANGMENT AT END OF WATERLINE EXTENSION

DWG. NO.
7

10/31/17

14

CONSTRUCTION DETAILS
S.R. 149

BEL-149-23.44

R32
R32

ITEM 632 - POWER SERVICE, AS PER PLAN

ELECTRIC POWER SHALL BE OBTAINED FROM THE AEP AT THE LOCATION INDICATED ON THE PLANS. POWER SUPPLIED SHALL BE 120 VOLTS.

POWER SERVICE SHALL BE AS PER CMS ITEM 632 AND SCD TC- 83.10 WITH THE FOLLOWING EXCEPTIONS:

1. THE CONTRACTOR SHALL MEET WITH A REPRESENTATIVE FROM THE POWER SUPPLY AGENCY TO CONFIRM HOW THE PROPOSED POWER SERVICE IS TO BE WIRED, HOOKED UP, AND ITS LOCATION

2. ALL POWER SERVICES SHALL BE METERED. THE METER SHALL HAVE A LEVER OPERATED BYPASS.

DISCONNECT SWITCH ENCLOSURES FURNISHED IN ACCORDANCE WITH CMS ITEM 632, POWER SERVICE, AS PER PLAN, SHALL INCLUDE A PADLOCK EQUAL TO MASTER NO. 4BKA OR WILSON BOHANNON 660, WITH LOCK BODY OF BRONZE OR BRASS AND KEYING SHALL BE TO THE STATE MASTER. THE CONTRACTOR SHALL CONTACT THE METER SECTION OF THE POWER COMPANY FOR INFORMATION REGARDING THE METER BASE INSTALLATION PRIOR TO ORDERING POLES. CONTRACTOR WILL BE RESPONSIBLE FOR REQUESTING AND SCHEDULING ANY INSPECTIONS THE POWER COMPANY MAY REQUIRE FOR THE POWER SERVICE HOOK UP. THE CONTRACTOR SHALL BE RESPONSIBLE TO CONTACT THE POWER COMPANY FOR THE ELECTRICAL SERVICE CONNECTION. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR SPLICE POWER CABLE INTO THE POWER COMPANY'S CIRCUITS. THE VOLTAGE SUPPLIED SHALL BE NOMINALLY 120 VOLTS. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ANY NECESSARY PERMITS AND THE PAYING OF ALL FEES. THE CONTRACTOR SHALL PAY ALL POWER CHARGES UNTIL THE SIGNAL IS ACCEPTED BY MAINTAINING AGENCY.

SIGNAL ACTIVATION

PRIOR TO ACTIVATION THE NEW TRAFFIC SIGNAL TO STOP-AND- GO MODE AND/OR REMOVING THE EXISTING TRAFFIC SIGNAL FROM SERVICE, ALL ITEMS IN THE PROPOSED SIGNAL PLAN SHALL BE FULLY COMPLETED, (I.E. VEHICLE DETECTION, PEDESTRIAN SIGNAL HEADS, ETC.). IF THERE ARE CONSTRUCTABILITY ISSUES (I.E., ROADWAY WIDENING, ETC.) THAT PREVENT THE SIGNAL FROM BEING COMPLETED PRIOR TO ACTIVATION, IT SHALL BE BROUGHT TO THE ATTENTION OF THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER. THE DISTRICT TRAFFIC ENGINEER WILL THEN REVIEW, APPROVE OR REJECT PROPOSALS TO ACTIVATE THE TRAFFIC SIGNAL PRIOR TO COMPLETION.

THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER AT LEAST 10 WORKING DAYS PRIOR TO SCHEDULING THE FINAL INSPECTION OF THE SIGNAL INSTALLATION. FINAL INSPECTION IS NOT CONSIDERED COMPLETE UNTIL DESIGNATED DISTRICT TRAFFIC PERSONNEL INSPECT THE TRAFFIC SIGNAL AND ISSUE WRITTEN APPROVAL. IF ISSUES ARE FOUND DURING THE FINAL INSPECTION THAT EFFECT THE SAFETY OF THE TRAVELING PUBLIC AND/OR THE EFFICIENCY OF THE INTERSECTION, THE SIGNAL SHALL NOT BE ACTIVATED OF THE PROPOSED DATE. ANY PUNCH LIST ITEMS THAT ARE FOUND SHALL BE CORRECTED AND REINSPECTED BY DISTRICT TRAFFIC PERSONNEL PRIOR TO FINAL ACCEPTANCE. ODOT FORCES SHALL ONLY ASSUME DAY TO DAY MAINTENANCE OF THE TRAFFIC SIGNAL AFTER FINAL WRITTEN ACCEPTANCE HAS BEEN ISSUED.

UNDERDRAINS FOR PULL BOXES

REFERENCE TRAFFIC SCD HL-30.11 FOR DETAILS ABOUT DRAINING PULL BOXES. UNDERDRAINS FOR PULL BOXES SHALL BE USED AS DIRECTED BY THE ENGINEER AND SHALL BE PROVIDED WHERE THE LENGTH REQUIRED FOR A SATISFACTORY OUTLET DOES NOT EXCEED 20 FEET. THE FOLLOWING ESTIMATED QUANTITY IS CARRIED TO THE GENERAL SUMMARY FOR THIS PURPOSE:

ITEM 611 - 4" CONDUIT, TYPE E FOR UNDERDRAIN OUTLET 10 FT

WORK INSPECTION

THE CONTRACTOR SHALL PROVIDE THE PROJECT ENGINEER AND THE DISTRICT TRAFFIC ENGINEER WITH 72 HOUR NOTICE OF ANY SIGNAL WORK TO BE PERFORMED AT THE INTERSECTION SITE(S) SO THAT INSPECTION SERVICES CAN BE SUPPLIED.

GUARANTEE

THE CONTRACTOR SHALL GUARANTEE THAT THE TRAFFIC CONTROL SYSTEM INSTALLED AS PART OF THIS CONTRACT SHALL OPERATE SATISFACTORILY FOR A PERIOD OF 90 DAYS FOLLOWING COMPLETION OF THE 10-DAY PERFORMANCE TEST. IN THE EVENT OF UNSATISFACTORY OPERATION THE CONTRACTOR SHALL CORRECT FAULTY INSTALLATIONS, MAKE REPAIRS AND REPLACE DEFECTIVE PARTS WITH NEW PARTS OF EQUAL OR BETTER QUALITY.

EQUIPMENT, MATERIAL AND LABOR COSTS INCURRED IN CORRECTING AN UNSATISFACTORY OPERATION SHALL BE BORNE BY THE CONTRACTOR.

THE GUARANTEE SHALL COVER THE FOLLOWING ITEMS OF THE TRAFFIC CONTROL SYSTEM: CONTROLLER, CABINET, UNINTERRUPTIBLE POWER SUPPLY, VEHICLE DETECTION EQUIPMENT, LED LAMP UNITS, NETWORK AND COMMUNICATION/INTERCONNECT EQUIPMENT.

CUSTOMARY MANUFACTURER'S GUARANTEES FOR THE FOREGOING ITEMS SHALL BE TURNED OVER TO ODOT DISTRICT 11 FOLLOWING ACCEPTANCE OF THE EQUIPMENT.

THE COST OF GUARANTEEING THE TRAFFIC CONTROL SYSTEM WILL BE INCIDENTAL TO AND INCLUDED IN THE CONTRACT UNIT PRICE OF THE VARIOUS ITEMS MAKING UP THE SYSTEM.

ITEM 625 - ARC FLASH CALCULATIONS AND LABEL

THIS ITEM SHALL INCLUDE PROVIDING ARC FLASH HAZARD CALCULATIONS AND LABELS PER SUPPLEMENTAL SPECIFICATION 825. LABELS SHALL BE APPLIED TO THE OUTSIDE OF EACH CONTROLLER CABINET. LABELS SHALL BE PROVIDED BY THE ODOT SIGN SHOP.

1606 WEST BROAD STREET
COLUMBUS, OHIO 43223

LOCATIONS OF ARC FLASH HAZARD LABELS ARE:

SIGNAL CONTROLLER (STA. 120+90.62, 36.58' LT.)

PAYMENT WILL BE MADE AT THE UNIT PRICE BID FOR EACH ITEM 625 "ARC FLASH CALCULATION LABEL" FOR EACH SET OF CALCULATIONS ACCEPTED AND LABEL INSTALLED, INCLUDING ALL LABOR, MATERIALS, AND INCIDENTALS NECESSARY.

ITEM 633 - CABINET, TYPE 332L, AS PER PLAN

THE CABINET SHALL BE FURNISHED AND INSTALLED ACCORDING TO CMS 633 AND 733, AND BE LISTED ON THE TRAFFIC AUTHORIZED PRODUCTS LIST (TAP).

THE CABINET SHALL BE FURNISHED WITH AN EDI MONITOR AS ALLOWED ON THE TAP/APPROVED PRODUCTS LIST. THE CABINET SHALL INCLUDE A CABINET RISER (12 INCH MINIMUM).

THE CONTRACTOR SHALL NOT REASSIGN THE CABINET DETECTOR INPUTS IN ORDER TO REDUCE THE NUMBER OF 2-CHANNEL DETECTOR UNITS SUPPLIED AND SHALL USE THE STANDARD CALTRANS INPUT FILE DESIGNATIONS FOLLOWING SHEET T6.

PAYMENT FOR ITEM 633 CABINET, TYPE 332L, AS PER PLAN WILL BE AT THE CONTRACT BID PRICE PER EACH COMPLETE AND IN PLACE INCLUDING ALL CONNECTIONS TESTED AND ACCEPTED.

ITEM 809 - ATC CONTROLLER, AS PER PLAN (PROGRAM AND INSTALL ONLY)

ALL REQUIREMENTS OF SS 809 SHALL BE FOLLOWED, ALONG WITH THE ADDITIONAL DESCRIPTION AS STATED BELOW. THE ATC CONTROLLER WILL BE PROVIDED BY THE DISTRICT WITHOUT PROGRAMMING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROGRAMMING THE CONTROLLER TO THE PROPOSED CONDITIONS ACCORDING TO THE PLANS. ODOT WILL NOT BE RESPONSIBLE FOR THE PROGRAMMING.

PAYMENT SHALL BE MADE ONCE THE CONTROLLER IS PROGRAMMED, INSTALLED, TESTED, FUNCTIONING ACCORDING TO THE PLANS, AND SHALL INCLUDE INCLUDE ALL LABOR, EQUIPMENT, MATERIALS, AND INCIDENTALS TO COMPLETE WORK.

CONTROLLER ITEM, MISC.: TIMING AND COORDINATION

THIS ITEM SHALL INCLUDE THE WORK NECESSARY TO CHANGE THE SIGNAL TIMING AND NETWORK COORDINATION IN THE LOCAL CONTROLLER LOCATED AT THE IR 70 WB RAMPS AS INDICATED ON SHEET T11.

COORDINATION TIMING WILL BE IMPLEMENTED AT THE FOLLOWING FOUR (4) INTERSECTIONS: S.R. 149 & RECO DRIVE, I-70 EASTBOUND RAMPS & S.R. 149, I-70 WESTBOUND RAMPS & S.R. 149, AND S.R. 149 & BOND DRIVE/SCHOOL STREET.

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TRAFFIC SIGNAL GENERAL NOTES
S.R. 149

BEL-149-23.44

T1
T13

ITEM 633 - UNINTERRUPTIBLE POWER SUPPLY (UPS), 1000 WATT, AS PER PLAN

IN ADDITION TO THE REQUIREMENTS OF C&MS 633 AND 733, POLE ATTACHMENT HARDWARE WILL BE INCLUDED FOR POLE-MOUNTED CABINETS, AND A CABINET RISER (8-INCH MINIMUM) AND ANCHOR BOLTS WILL BE PROVIDED FOR BASE-MOUNTED CABINETS. BEFORE PERFORMING THE WORK, THE CONTRACTOR, THE DISTRICT TRAFFIC ENGINEER, AND THE PROJECT ENGINEER WILL PERFORM A SITE INSPECTION TO ESTABLISH THE LOCATION OF THE UPS CABINET AND FOUNDATION.

THE UPS CABINET SHALL INCLUDE A GENERATOR POWER PANEL WITH A HEAVY-DUTY POWER RELAY VERSUS THE LINE VOLTAGE GENERATOR SWITCH. THE GENERATOR INLET SHALL BE A RECESSED PANEL WITH A DOOR THAT IS FLUSH WITH THE EXTERNAL SIDE OF THE UPS CABINET. IT SHALL INCLUDE A RECESSED PLUG, AUTOMATIC TRANSFER SWITCH AND A DOOR THAT SECURELY CLOSES OVER THE POWER CORD.

THE CABINET SHALL HAVE A DOOR STOP MECHANISM AND THERMOSTATICALLY CONTROLLED FAN.

THE CABINET SHALL INCLUDE A BATTERY BALANCING DEVICE THAT REGULATES THE BATTERIES AND OPTIMIZES PERFORMANCE.

AFTER FOUR (4) HOURS OF BATTERY RUNTIME, THE SYSTEM SHALL BE PROGRAMMED TO SWITCH THE INTERSECTION FROM FULL OPERATION TO CONTROLLER AUTOMATIC FLASH OPERATION THROUGH THE MONITOR. THE CONTROLLER SHALL BE PROGRAMMED SO THAT FLASH OPERATION SHALL BEGIN ONCE THE INTERSECTION RUNS MINOR STREET GREEN (TYP. PH. 4 & 8), ALL-RED CLEARANCE, AND THEN FLASH OPERATION.

THE UPS OUTPUT NOTIFICATIONS FOR ON BATTERY, BATTERY 2-HOUR TIMER, AND LOW BATTERY SHALL BE WIRED INTO THE TRAFFIC SIGNAL CABINET BACK PANEL OR THROUGH THE CONTROLLER WITH A C11 TO PROVIDE SPECIAL STATUS ALARMS FOR EACH OUTPUT INTO THE SIGNAL CONTROLLER.

THIS ITEM SHALL INCLUDE A RED LED STATUS INDICATOR LAMP TO ALLOW MAINTENANCE PERSONNEL AND LAW ENFORCEMENT TO QUICKLY ASSESS WHETHER A TRAFFIC SIGNAL CABINET IS BEING POWERED BY A UPS. THE LED HOUSING SHALL BE NEMA 4X, IP65 OR IP66, RATED FOR OUTDOOR USE AND BE TAMPER/SHATTER RESISTANT. IT SHALL BE A DOMED ENCLOSURE CONTAINING A RED LENS WITH LED THAT IS VISIBLE FROM 100 FOOT MINIMUM. THE ENCLOSURE AND LED MODULE SHOULD BE PLACED ON THE SIDE OF THE UPS CABINET FACING TOWARDS THE MAINLINE ROADWAY AND SEALED FROM WATER INTRUSION. IT SHOULD BE WIRED USING MINIMUM 20GA STRANDED, INSULATED HOOKUP WIRE TO THE STATUS RELAY OUTPUTS OF THE UPS. THE WIRES SHALL BE TERMINATED BY LUGS AT THE DISPLAY END AND PERMANENTLY LABELED "BACKUP POWER STATUS DISPLAY," WITH WIRE POLARITY INDICATED. THE RED LED SHALL ONLY ILLUMINATE TO INDICATE THE CABINET IS OPERATING UNDER UPS BACKUP POWER (THE "BACKUP" OPERATING CONDITION). THIS ITEM INCLUDES PROGRAMMING THE UPS STATUS RELAY OUTPUTS TO PRODUCE THE LAMP STATUS DISPLAYS. THESE STATUS DISPLAYS WILL BE SOLID 100% DUTY CYCLE (NOT FLASHING) DISPLAYS. THE OPERATING VOLTAGE OF THE LED LAMP SHALL BE 120V AC UNLESS OTHERWISE INDICATED.

ITEM 809 - ADVANCE RADAR DETECTION, AS PER PLAN

THIS ITEM OF WORK SHALL CONSIST OF FURNISHING AND INSTALLING A WAVETRONIX SMARTSENSOR ADVANCE DETECTION UNTIL (MODEL SS-200E). THE DETECTION UNIT SHALL INCLUDE THE FOLLOWING:

1. POWER SHALL BE PROVIDED FROM THE TRAFFIC CABINET.
2. ALL REQUIRED INPUTS CARDS SHALL BE INCLUDED IN THE TRAFFIC CABINET AND SHALL BE COMPATIBLE WITH CALTRANS, NEMA TS1 AND NEMA TS2 DETECTOR RACKS. THE CARDS SHALL PROVIDE TRUE PRESENCE DETECTOR CALLS OR CONTACT CLOSURE TO THE TRAFFIC CONTROLLER.
3. THE UNIT SHALL BE MOUNTED DIRECTLY TO A POLE OR MAST ARM, AS RECOMMENDED BY THE MANUFACTURER. CABLE(S) SHALL BE PROVIDED AS REQUIRED AND RECOMMENDED BY THE MANUFACTURER.
4. SURGE PROTECTION DEVICES, AS RECOMMENDED BY THE MANUFACTURER SHALL BE INCLUDED BOTH AT THE POLE WHERE THE UNIT IS LOCATED TO PROTECT THE UNIT AND IN THE TRAFFIC CABINET TO PROTECT THE CABINET ELECTRONICS.
5. THE MANUFACTURER'S REPRESENTATIVE SHALL BE ON SITE DURING INSTALLATION AND TESTING AND SHALL PROVIDE ONSITE TRAINING ON THE SETUP, OPERATION AND MAINTENANCE OF THE UNIT.
6. A SERIAL TO ETHERNET COMMUNICATIONS MODULE AND ETHERNET CABLE (MINIMUM 7 FEET).
7. THE POWER SUPPLY AND COMMUNICATION MODULES SHALL BE SECURED TO A SINGLE PANEL THAT CAN BE MOUNTED INTERIOR TO THE TRAFFIC CABINET. THE PANEL SHALL INCLUDE MODULAR-PLUG STYLE CONNECTIONS FOR UP TO FOUR (4) SENSOR CABLES. ADDITIONAL SENSORS MAY BE HARD-WIRED TO THE COMMUNICATION MODULES, AS NECESSARY.
8. THE CONTRACTOR SHALL INSTALL THE RADAR DETECTION PRIOR TO MILLING/DISABLING EXISTING LOOPS.
9. THE INSTALLATION SHALL INCLUDE ALL CONTROLLER PROGRAMMING FOR COMPLETE INSTALLATION, WHICH INCLUDES MODIFICATIONS FOR REMOVAL OF EXISTING DETECTION.

PAYMENT FOR ITEM 809 ADVANCE RADAR DETECTION, AS PER PLAN SHALL BE MADE AT THE CONTRACT UNIT PRICE FOR EACH UNIT, COMPLETE AND IN PLACE INCLUDING ALL REQUIRED CABINET HARDWARE, MOUNTING BRACKETS, CABLES, CONDUIT, CONNECTIONS TESTED AND ACCEPTED, AND ANY OTHER NECESSARY HARDWARE TO ESTABLISH A FULLY FUNCTIONAL DETECTION SYSTEM.

DETECTION MAINTENANCE

IF VEHICLE DETECTION BECOMES UNEXPECTEDLY DISABLED, REQUIRES MODIFICATION, OR IS SCHEDULED TO BE TEMPORARILY REMOVED DURING THE CONSTRUCTION PROJECT, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE PROJECT ENGINEER AND DISTRICT TRAFFIC ENGINEER.

IF THE LOSS OF VEHICLE DETECTION IS KNOWN PRIOR TO THE START OF CONSTRUCTION, IT SHALL BE DISCUSSED AT THE PRECONSTRUCTION MEETING. AT SUCH TIME, THE DISTRICT TRAFFIC ENGINEER SHALL ADVISE THE PROJECT ENGINEER AND CONTRACTOR ON THE APPROPRIATE ACTION TO RECTIFY ANY LOSS OF VEHICLE DETECTION. THIS MAY INCLUDE PLACING THE TRAFFIC SIGNAL ON MINIMUM OR MAXIMUM RECALL, MODIFYING THE MINIMUM GREEN TIMES, AND REMOVING THE MALFUNCTIONING DETECTION FROM SERVICE. WHERE NON-INTRUSIVE DETECTION (I.E. VIDEO, RADAR) ALREADY EXISTS, THE CONTRACTOR SHALL INSURE THAT DETECTION IS OPERATING AND MAINTAINED BY RECONFIGURING THE DETECTION UNITS ACCORDINGLY DURING ALL CONSTRUCTION PHASES. THIS IS TO AVOID THE SIGNAL FROM MAXING OUT THE EFFECTED SIGNAL PHASE AND CREATING UNNECESSARY DELAYS.

LOCATIONS WHERE NON-INTRUSIVE DETECTION IS PROPOSED AND THE EXISTING VEHICLE DETECTION IS TO BE ABANDON, THE NON-INTRUSIVE VEHICLE DETECTION SHALL BE INSTALLED, CONFIGURED AND MADE FULLY FUNCTIONAL PRIOR TO THE EXISTING DETECTION BEING DISABLED. THE CONTRACTOR SHALL CONTINUE TO MAINTAIN AND MODIFY THE DETECTION UNTIL FINAL ACCEPTANCE OF THE TRAFFIC SIGNAL. THIS IS TO ENSURE VEHICLE DETECTION REMAINS FULLY FUNCTIONAL THROUGHOUT CONSTRUCTION.

ITEM 633 - COMMUNICATIONS, AS PER PLAN

FURNISH A CELLULAR MODEM, ONE 3-ANTENNA ASSEMBLY (PART #6001136), AND A 10' ETHERNET CABLE FOR REMOTE WIRELESS CELLULAR COMMUNICATION.

FOR NETWORK CONSISTENCY CELLULAR MODEMS SHALL BE THE SIERRA WIRELESS:
MODEM, AIRLINK MP70 ETHERNET WITH AC TO DC POWER CABLE - MODEL 1102709KIT

THIS ITEM SHALL INCLUDE THE FURNISHING A MOUNTING BRACKET FOR THE ANTENNA WITH ALL NECESSARY HARDWARE INCLUDING BUT NOT LIMITED TO SPRING NUTS, WASHERS, AND BOLTS THAT INSTALLS TO THE MOUNTING CHANNEL ON THE SIDE OF THE SIGNAL CABINET.

THE CELLULAR MODEM EQUIPMENT SHALL BE DELIVERED TO ODOT DISTRICT 11 TRAFFIC FOR PROGRAMMING AND INSTALLATION.

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2201 REISER AVENUE SE
NEW PHILADELPHIA, OHIO 44663

THE CONTRACTOR SHALL PROVIDE THE MODEM SERIAL NUMBERS AND NECESSARY ESN NUMBERS FOR ODOT TO ESTABLISH WIRELESS SERVICE.

THE DEPARTMENT WILL MEASURE "COMMUNICATIONS, AS PER PLAN" BY THE NUMBER OF COMPLETE UNITS FURNISHED, RECEIVED, AND ACCEPTED BY ODOT DISTRICT 11 TRAFFIC.

ITEM 809 - STOP LINE RADAR DETECTION, AS PER PLAN

THIS ITEM OF WORK SHALL CONSIST OF FURNISHING AND INSTALLING A WAVETRONIX SMARTSENSOR MATRIX DETECTION UNIT. THE DETECTION UNIT SHALL INCLUDE THE FOLLOWING:

1. POWER SHALL BE PROVIDED FROM THE TRAFFIC CABINET.
2. ALL REQUIRED INPUTS CARDS SHALL BE INCLUDED IN THE TRAFFIC CABINET AND SHALL BE COMPATIBLE WITH CALTRANS, NEMA TS1 AND NEMA TS2 DETECTOR RACKS. THE CARDS SHALL PROVIDE TRUE PRESENCE DETECTOR CALLS OR CONTACT CLOSURE TO THE TRAFFIC CONTROLLER.
3. THE UNIT SHALL BE MOUNTED DIRECTLY TO A POLE OR MAST ARM, AS RECOMMENDED BY THE MANUFACTURER. CABLE(S) SHALL BE PROVIDED AS REQUIRED AND RECOMMENDED BY THE MANUFACTURER.
4. SURGE PROTECTION DEVICES, AS RECOMMENDED BY THE MANUFACTURER SHALL BE INCLUDED BOTH AT THE POLE WHERE THE UNIT IS LOCATED TO PROTECT THE UNIT AND IN THE TRAFFIC CABINET TO PROTECT THE CABINET ELECTRONICS.
5. THE MANUFACTURER'S REPRESENTATIVE SHALL BE ON SITE DURING INSTALLATION AND TESTING AND SHALL PROVIDE ONSITE TRAINING ON THE SETUP, OPERATION AND MAINTENANCE OF THE UNIT.
6. A SERIAL TO ETHERNET COMMUNICATIONS MODULE AND ETHERNET CABLE (MINIMUM 7 FEET).
7. THE POWER SUPPLY AND COMMUNICATION MODULES SHALL BE SECURED TO A SINGLE PANEL THAT CAN BE MOUNTED INTERIOR TO THE TRAFFIC CABINET. THE PANEL SHALL INCLUDE MODULAR-PLUG STYLE CONNECTIONS FOR UP TO FOUR (4) SENSOR CABLES. ADDITIONAL SENSORS MAY BE HARD-WIRED TO THE COMMUNICATION MODULES, AS NECESSARY.
8. THE CONTRACTOR SHALL INSTALL THE RADAR DETECTION PRIOR TO MILLING/DISABLING EXISTING LOOPS.
9. THE INSTALLATION SHALL INCLUDE ALL CONTROLLER PROGRAMMING FOR COMPLETE INSTALLATION, WHICH INCLUDES MODIFICATIONS FOR REMOVAL OF EXISTING DETECTION.

PAYMENT FOR ITEM 809 STOP-LINE RADAR DETECTION, AS PER PLAN SHALL BE MADE AT THE CONTRACT UNIT PRICE FOR EACH UNIT, COMPLETE AND IN PLACE INCLUDING ALL REQUIRED CABINET HARDWARE, MOUNTING BRACKETS, CABLES, CONDUIT AND CONNECTIONS TESTED AND ACCEPTED.

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TRAFFIC SIGNAL GENERAL NOTES
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GROUNDING AND BONDING

THE REQUIREMENTS OF THE CONSTRUCTION AND MATERIAL SPECIFICATIONS (C&MS) AND THE TC SERIES OF STANDARD CONSTRUCTION DRAWINGS ARE MODIFIED AS FOLLOWS:

1. ALL METALLIC PARTS CONTAINING ELECTRICAL CONDUCTORS SHALL BE PERMANENTLY JOINED TO FORM AN EFFECTIVE GROUND FAULT CURRENT PATH BACK TO THE GROUNDED CONDUCTOR IN THE POWER SERVICE DISCONNECT SWITCH.
 - A. PROVIDE AN EQUIPMENT GROUNDING CONDUCTOR IN METALLIC CONDUITS (725.04) IN ADDITION TO THE CONDUCTORS SPECIFIED AND BOND THE CONDUIT TO THIS GROUNDING CONDUCTOR.
 - B. WHEN AN EQUIPMENT GROUNDING CONDUCTOR IS REQUIRED IN PLASTIC CONDUIT (725.05), THE INSTALLATION SHALL INCLUDE A SEPARATE EQUIPMENT GROUNDING CONDUCTOR IN ADDITION TO THE CONDUCTORS SPECIFIED.
 - C. METALLIC CONDUIT CARRYING THE LOOP WIRES FROM IN THE PAVEMENT TO THE PULL BOX SPLICE LOCATION WILL ONLY BE BONDED AT THE PULL BOX END, AND WILL NOT CONTAIN AN EQUIPMENT GROUNDING CONDUCTOR.
 - D. IF MULTIPLE CONDUIT RUNS BEGIN AND END AT THE SAME POINTS, ONLY ONE EQUIPMENT GROUNDING CONDUCTOR IS REQUIRED.
 - E. IF AN EQUIPMENT GROUNDING CONDUCTOR IS NEEDED IN CONDUIT BETWEEN SIGNALIZED INTERSECTIONS FOR UNDERGROUND INTERCONNECT CABLE, THE GROUNDING SYSTEM FOR EACH SIGNALIZED INTERSECTION WILL BE SEPARATED ABOUT MIDWAY BETWEEN THE INTERSECTIONS.
 - F. THE MESSENGER WIRE AT SIGNALIZED INTERSECTIONS WILL BE USED AS THE CONDUCTIVE PATH FROM CORNER TO CORNER IF CONDUIT IS NOT PROVIDED UNDER THE ROADWAY. WHEN CONDUIT CONNECTS THE CORNERS OF AN INTERSECTION, AN EQUIPMENT GROUNDING CONDUCTOR SHALL BE USED IN THE CONDUIT.

2. CONDUITS

- A. THE 725.04 CONDUIT SHALL HAVE GROUNDING BUSHINGS INSTALLED AT ALL TERMINATION POINTS. THE BUSHING MATERIAL SHALL BE COMPATIBLE WITH GALVANIZED STEEL CONDUIT AND THE GROUNDING LUG MATERIAL SHALL BE COMPATIBLE FOR USE WITH COPPER WIRE. THREADED OR COMPRESSION TYPE BUSHINGS MAY BE USED.
- B. THE 725.05 CONDUIT SHALL HAVE THE INSIDE AND OUTSIDE DIAMETERS OF THE CONDUIT DEBURRED AT ALL TERMINATION POINTS.
- C. BOTH ENDS OF METALLIC CONDUIT SHALL BE BONDED TO THE EQUIPMENT GROUNDING CONDUCTOR.
- D. METALLIC CONDUIT MAY BE BONDED TO METALLIC BOXES THROUGH THE USE OF CONDUIT FITTINGS UL APPROVED FOR THIS TYPE OF CONNECTION, WITH THE BOX BONDED TO THE EQUIPMENT GROUNDING CONDUCTOR.

GROUNDING AND BONDING (CONT.)

3. WIRE FOR GROUNDING AND BONDING.

- A. USE INSULATED, COPPER WIRE FOR THE EQUIPMENT GROUNDING CONDUCTOR. BONDING JUMPERS IN BOXES AND ENCLOSURES MAY BE BARE OR INSULATED COPPER WIRE. WIRE SIZE SHALL BE AS FOLLOWS:
 - I. USE 4 AWG BETWEEN THE POWER SERVICE AND SUPPORTS, POLES, PEDESTALS, CONTROLLER OR FLASHER CABINETS.
 - II. USE A MINIMUM 8 AWG BETWEEN LOOP DETECTOR PULL BOXES AND THE FIRST CONDUIT THAT REQUIRES A LARGER SIZE AS SPECIFIED IN 3.A.I ABOVE.
 - III. USE A MINIMUM 8 AWG BETWEEN THE "PREPARE TO STOP WHEN FLASHING" INSTALLATION (INCLUDING SUPPORT) AND THE FIRST CONDUIT THAT REQUIRES A LARGER SIZE AS SPECIFIED IN 3.A.I ABOVE.
 - IV. THE INSULATION SHALL BE GREEN OR GREEN WITH YELLOW STRIPE(S). FOR 4 AWG OR LARGER, INSULATION MAY ALSO BE BLACK WITH GREEN TAPE/LABELS INSTALLED AT ALL ACCESS POINTS.
- B. IN A HIGHWAY LIGHTING SYSTEM, THE EQUIPMENT GROUNDING CONDUCTOR SHALL BE THE SAME WIRE SIZE AS THE DUCT CABLE OR DISTRIBUTION CABLE CIRCUIT CONDUCTORS, WITH THE MINIMUM CONDUCTOR SIZE OF 4 AWG. BONDING JUMPERS WILL BE MINIMUM SIZE 4 AWG.

4. GROUND ROD

- A. A 3/4 INCH SCHEDULE 40 PVC CONDUIT WILL BE USED IN FOUNDATIONS AND CONCRETE WALLS FOR THE GROUNDING CONDUCTOR (GROUND WIRE) RACEWAY TO THE GROUND ROD. SHOULD METALLIC CONDUIT BE USED, BOTH ENDS OF THE CONDUIT SHALL BE BONDED TO THE GROUNDING CONDUCTOR.
- B. THE TYPICAL GROUNDING CONDUCTOR (GROUND WIRE) SHALL BE 4 AWG INSULATED, COPPER.

5. THE GREEN CONDUCTOR IN SIGNAL CABLES (CONDUCTOR #4) SHALL NOT BE USED TO SUPPLY POWER TO A SIGNAL INDICATION. IT WILL BE CONNECTED TO THE SIGNAL BODY AS AN EQUIPMENT GROUND IN ALUMINUM HEADS AND IT WILL BE UNUSED IN PLASTIC HEADS. UNUSED CONDUCTORS SHALL BE GROUNDED IN THE CABINET. TYPICAL USE OF CONDUCTORS IS AS FOLLOWS:

COND. NO	COLOR	VEHICLE SIGNAL	PEDESTRIAN SIGNAL
1	BLACK	GREEN BALL	#1 WALK
2	WHITE	AC NEUTRAL	AC NEUTRAL
3	RED	RED BALL	#1 DW/FDW
4	GREEN	EQUIPMENT GROUND	EQUIPMENT GROUND
5	ORANGE	YELLOW BALL	#2 DW/FDW
6	BLUE	GREEN ARROW	#2 WALK
7	WHITE/BLACK STRIPE	YELLOW ARROW	NOT USED

GROUNDING AND BONDING (CONT.)

6. POWER SERVICE AND DISCONNECT SWITCH.

- A. AT THE POWER SERVICE LOCATION, THE GROUNDING CONDUCTOR (GROUND WIRE) FROM THE DISCONNECT SWITCH NEUTRAL (AC-) BAR TO THE GROUND ROD SHALL BE A CONTINUOUS, UNSPLICED CONDUCTOR. IF SPLICED, IT SHALL BE AN EXOTHERMIC WELD BUTT SPLICE.
 - B. THE SERVICE NEUTRAL (AC-) SHALL ONLY BE CONNECTED TO GROUND AT THE PRIMARY POWER SERVICE DISCONNECT SWITCH.
 - I. NEMA CONTROLLER CABINETS: IF A POWER SERVICE DISCONNECT SWITCH IS LOCATED BEFORE THE CONTROLLER CABINET, THE NEUTRAL (AC-) AND THE GROUNDING BARS IN THE CONTROLLER CABINET SHALL NOT BE CONNECTED TOGETHER AS SHOWN IN NEMA TS-2, FIGURE 5-4.
 - II. IF SECONDARY DISCONNECT SWITCHES ARE CONNECTED AFTER THE PRIMARY DISCONNECT SWITCH, THE NEUTRAL (AC-) SHALL ONLY BE GROUNDED AT THE PRIMARY SWITCH. EQUIPMENT GROUNDING CONDUCTORS SHALL BE BROUGHT TO THE PRIMARY SWITCH, BUT SHALL BE GROUNDED AT BOTH SECONDARY AND PRIMARY SWITCHES.
7. PAYMENT - ALL MATERIALS AND WORK REQUIRED TO COMPLETE THE EFFECTIVE GROUND FAULT CURRENT PATH SYSTEM ARE INCIDENTAL TO THE CONDUCTORS INSTALLED BY CONTRACT.

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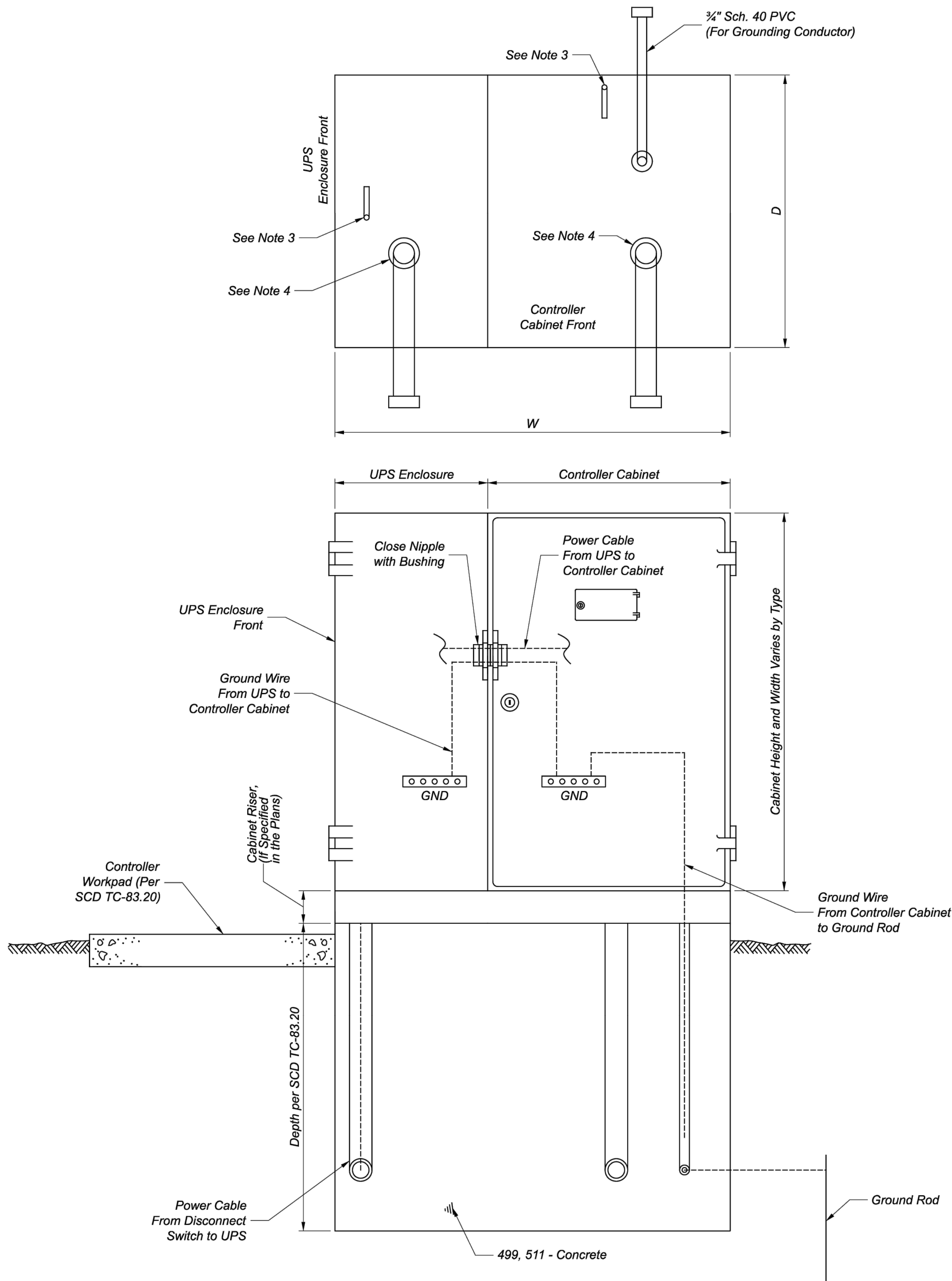
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TRAFFIC SIGNAL GENERAL NOTES
S.R. 149

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NOTES:

1. THE UNINTERRUPTIBLE POWER SUPPLY (UPS) ENCLOSURE SHALL BE MOUNTED FLUSH UP AGAINST THE TRAFFIC SIGNAL CABINET AND SEALED WITH SILICONE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE NECESSARY POWER CABLE BETWEEN THE UPS UNIT AND SIGNAL CABINET.
2. THE UPS SHOULD BE PLACED ON THE OPPOSITE SIDE OF THE PULL BOX ON A 332/336 CABINET (PER STANDARD CONSTRUCTION DRAWING (SCD) TC-83.20). THE UPS PLACEMENT FOR A NEMA CABINET VARIES, PLACEMENT SHOULD PROVIDE ADEQUATE ACCESS WITH RESPECT TO SLOPE, GUARDRAIL SPACING, ETC.
3. THE SIZE, NUMBER, AND LOCATION OF ANCHOR BOLTS SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
4. THE SIZE, NUMBER, AND ORIENTATION OF CONDUIT ELLS SHALL BE AS SHOWN IN THE PLAN, EXCEPT THAT A ¾" SCHEDULE 40 PVC SHALL BE INSTALLED IN EACH FOUNDATION.
5. ½" PREFORMED JOINT FILLER AS PER CMS 705.03 SHALL BE USED BETWEEN FOUNDATIONS AND ADJACENT PAVED AREAS.
6. SEE SCD TC-83.20 FOR FURTHER DETAILS.

TYPE	W (IN.)	D (IN.)	FOUNDATION CONCRETE (CU. YD.)
TS-1	60	24	1.23
TS-2	70	36	2.16
2070/170	50	36	1.54

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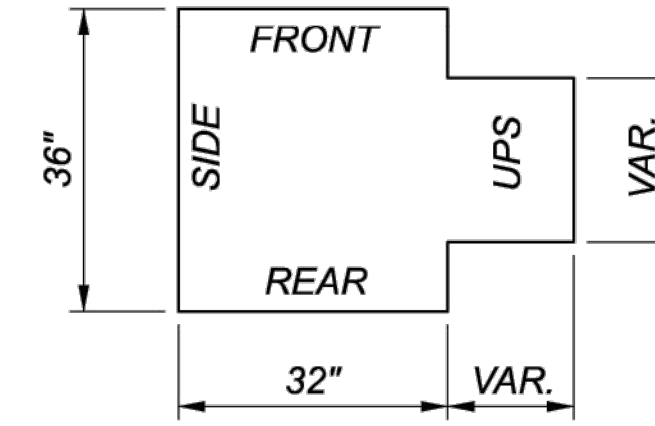
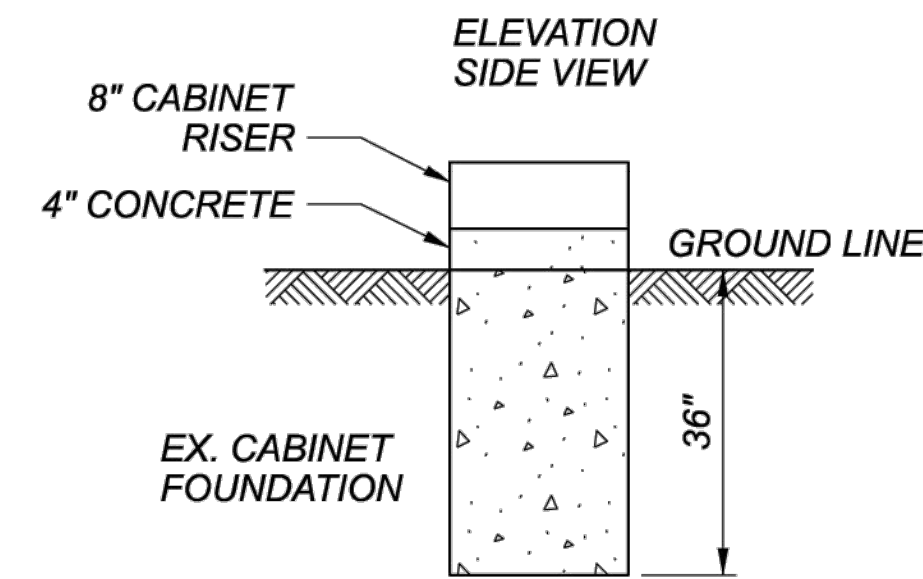
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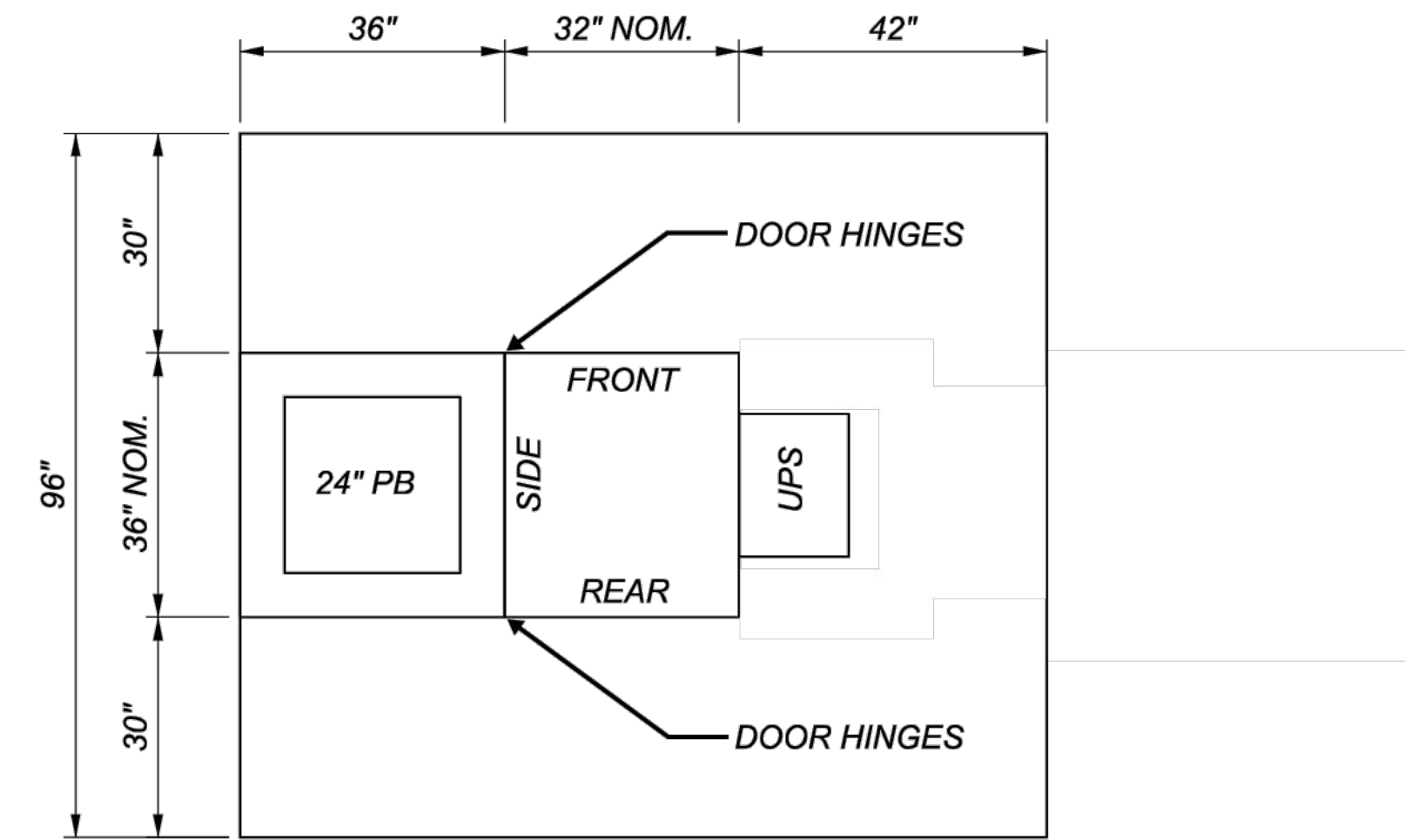
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MODEL 332 CABINET DETAIL (TYP.)

UPS FOUNDATION DETAIL



UPS WORK PAD DETAIL



PLAN VIEW

NOTES:

- 1) THE SIZE OF THE UPS FOUNDATION MAY VARY BASED ON THE CABINET SIZE PROVIDED.
- 2) UPS FOUNDATION ELEVATION SHOULD MATCH CABINET FOUNDATION ELEVATION.
- 3) THE UPS CABINET SHALL BE MOUNTED FLUSH UP AGAINST THE SIGNAL CABINET AND SEALED.
- 4) CONDUIT AND WIRING FROM THE SIGNAL CABINET TO THE UPS SHALL BE INSTALLED THROUGH THE CABINET RISER.

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SEPAC AND ASC/3 INPUT FILE INFORMATION FOR THE 332 CABINET

UPPER INPUT FILE (FILE=I)

C U P P E R L	PHASE	1	2	2	2	3	4	4	4	1	SPARE	MANUAL CONTROL ADV.	2	6	FLASH
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL			PED	PED	SENSE
	SEPAC DETECTOR NO.	VEH 1	VEH 3	VEH 5	VEH 7	VEH 9	VEH 11	VEH 13	VEH 15	VEH 17			PED 2	PED 6	
	ASC/3 DETECTOR NO.	VEH 1	VEH 2	VEH 3	VEH 4	VEH 5	VEH 6	VEH 7	VEH 8	VEH 9			PED 2	PED 6	
	C1 PIN NUMBER	56	39	63	47	58	41	65	49	60		80	67	68	81
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E	
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14
C L O W E R L	PHASE	1	2	2	2	3	4	4	4	3	SPARE	ADV. ENABLE	4	8	STOP
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL			PED	PED	TIME
	SEPAC DETECTOR NO.	VEH 1	VEH 4	VEH 6	VEH 7	VEH 9	VEH 12	VEH 14	VEH 15	VEH 18			PED 4	PED 8	
	ASC/3 DETECTOR NO.	VEH 1	VEH 10	VEH 11	VEH 4	VEH 5	VEH 14	VEH 15	VEH 8	VEH 13			PED 4	PED 8	
	C1 PIN NUMBER	56	43	76	47	58	45	78	49	62		53	69	70	82
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K	

LOWER INPUT FILE (FILE=J)

C U P P E R L	PHASE	5	6	6	6	7	8	8	8	5	SPARE	SPARE	EV - A	EV - B	RR - 1
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL					
	SEPAC DETECTOR NO.	VEH 19	VEH 21	VEH 23	VEH 25	VEH 29	VEH 31	VEH 33	VEH 35	VEH 37					
	ASC/3 DETECTOR NO.	VEH 17	VEH 18	VEH 19	VEH 20	VEH 21	VEH 22	VEH 23	VEH 24	VEH 25					
	C1 PIN NUMBER	55	40	64	48	57	42	66	50	59		54	71	72	51
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E	
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14
C L O W E R L	PHASE	5	6	6	6	7	8	8	8	7	SPARE	SPARE	EV - C	EV - D	RR - 2
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL					
	SEPAC DETECTOR NO.	VEH 19	VEH 22	VEH 24	VEH 25	VEH 29	VEH 32	VEH 34	VEH 35	VEH 38					
	ASC/3 DETECTOR NO.	VEH 17	VEH 26	VEH 27	VEH 20	VEH 21	VEH 30	VEH 31	VEH 24	VEH 29					
	C1 PIN NUMBER	55	44	77	48	57	46	79	50	61		75	73	74	52
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K	

SEPAC AND ASC/3 INPUT FILE INFORMATION FOR THE 336 CABINET

C U P P E R L	PHASE	1	2	3	4	5	6	7	8	RR - 1	EV - A	EV - B	2	6	FLASH
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL				PED	PED	SENSE
	SEPAC DETECTOR NO.	VEH 1	VEH 3	VEH 9	VEH 11	VEH 19	VEH 21	VEH 29	VEH 31				PED 2	PED 6	
	ASC/3 DETECTOR NO.	VEH 1	VEH 2	VEH 5	VEH 6	VEH 17	VEH 18	VEH 21	VEH 22				PED 2	PED 6	
	C1 PIN NUMBER	56	39	58	41	55	40	57	42	51	71	72	67	68	81
FIELD TERMINALS	1-D,E	2-D,E	3-D,E	4-D,E	5-D,E	6-D,E	7-D,E	8-D,E	9-D,E	10-D,E	11-D,E	12-D,E	13-D,E	14-D,E	
SLOT NUMBER		1	2	3	4	5	6	7	8	9	10	11	12	13	14
C L O W E R L	PHASE	2	2	4	4	6	6	8	8	RR - 2	EV - C	EV - D	4	8	STOP
	DEFAULT FUNCTION	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL	EXT-CALL				PED	PED	TIME
	SEPAC DETECTOR NO.	VEH 7	VEH 4	VEH 15	VEH 12	VEH 25	VEH 22	VEH 35	VEH 32				PED 4	PED 8	
	ASC/3 DETECTOR NO.	VEH 4	VEH 10	VEH 8	VEH 14	VEH 20	VEH 26	VEH 24	VEH 30				PED 4	PED 8	
	C1 PIN NUMBER	47	43	49	45	48	44	50	46	52	73	74	69	70	82
FIELD TERMINALS	1-J,K	2-J,K	3-J,K	4-J,K	5-J,K	6-J,K	7-J,K	8-J,K	9-J,K	10-J,K	11-J,K	12-J,K	13-J,K	14-J,K	

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TRAFFIC SIGNAL GENERAL NOTES
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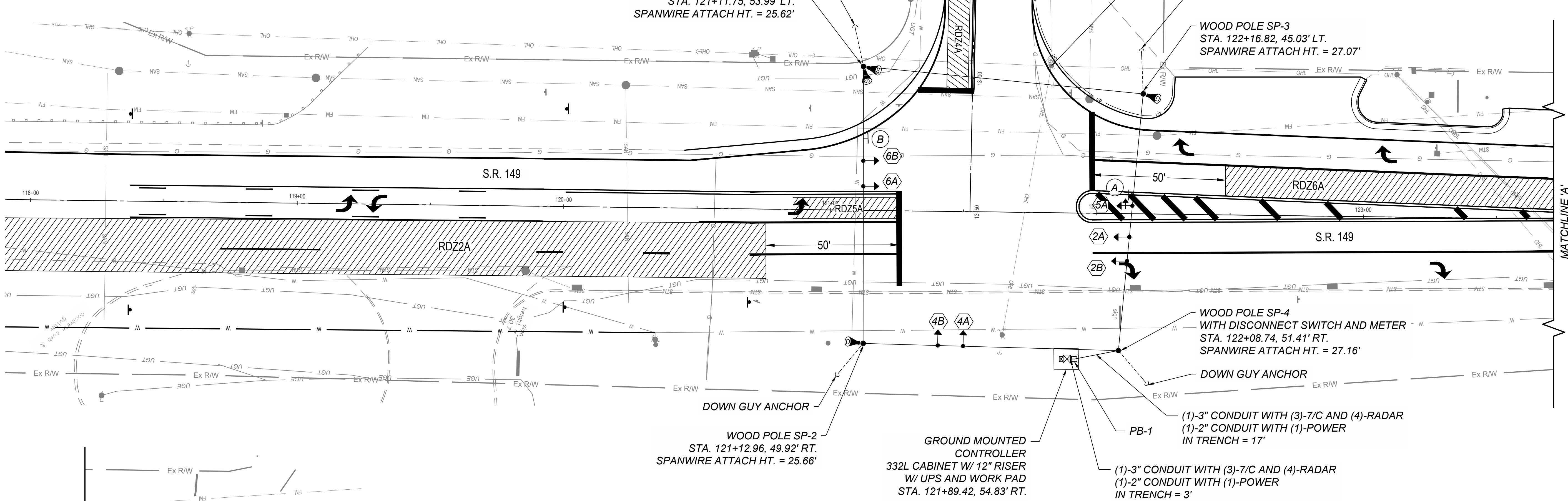
NOTES:

THE CONTRACTOR SHALL FIELD VERIFY THE LOCATIONS OF ALL UTILITIES PRIOR TO EXCAVATION.

THE CONTRACTOR SHALL COORDINATE WITH UTILITY PROVIDER TO PROVIDE AND INSTALL TEMPORARY POWER SERVICE ON WOOD POLE SP-4.

THE CONTRACTOR/UTILITY COMPANY SHALL MAINTAIN PROPER HORIZONTAL AND VERTICAL CLEARANCES FROM OVERHEAD UTILITIES TO WOOD POLE SP-3 WHEN RELOCATING THE EXISTING UTILITY POLE AND OVERHEAD LINES AT THE NW CORNER OF THE INTERSECTION.

THE CONTRACTOR SHALL USE A TETHER WIRE IN ADDITION TO THE MESSENGER WIRE TO SECURE SIGNAL HEADS W/ BACKPLATES AND SIGNAGE.

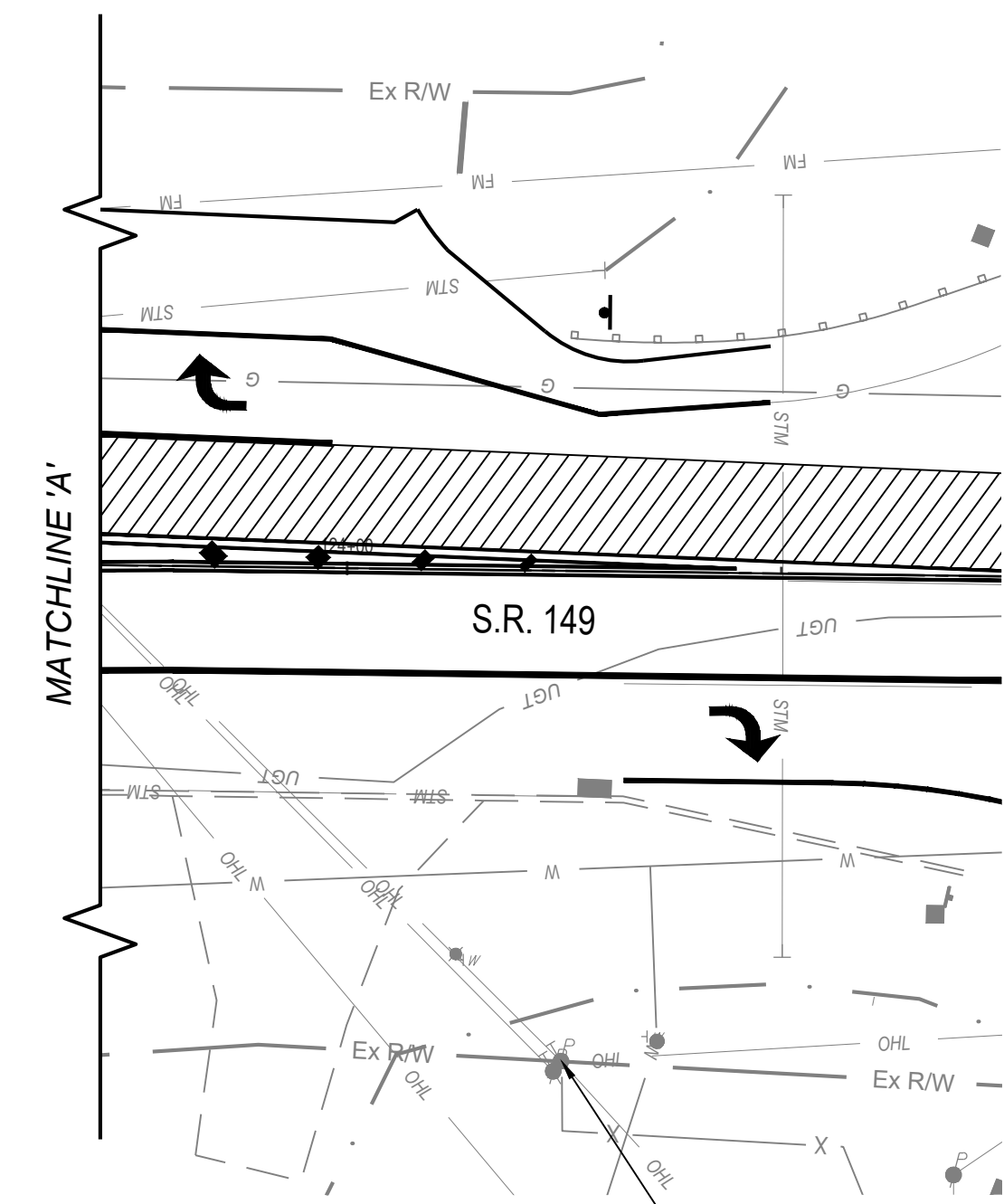


PULL BOX DATA			
NO.	STATION	OFFSET	SIZE
PB-1	121+92.25	54.77' RT.	24"



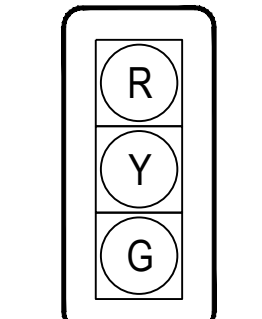
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TRAFFIC SIGNAL PLAN
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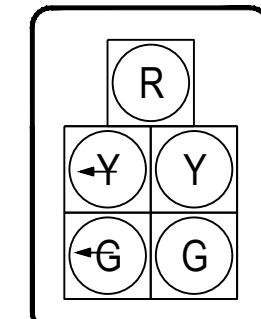


PR. POWER SERVICE WITH 2" CONDUIT RISER STA. 124+25.34, 56.48' RT

PROPOSED VEHICULAR SIGNAL HEADS



SIGNALS
2A 2B 4A
4B 6A 6B

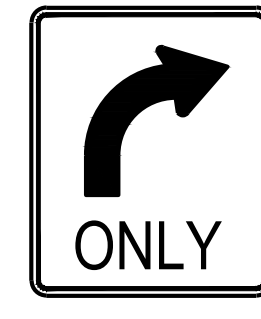


SIGNALS
5A

- ALL SIGNAL HEADS SHALL HAVE 12" LED LENSES.
- ALL SIGNAL HEADS SHALL BE BLACK IN COLOR AND HAVE BACKPLATES.
- ALL SIGNAL HEAD VISORS SHALL BE TUNNEL TYPE.



R3-5L
30" x 36"
A

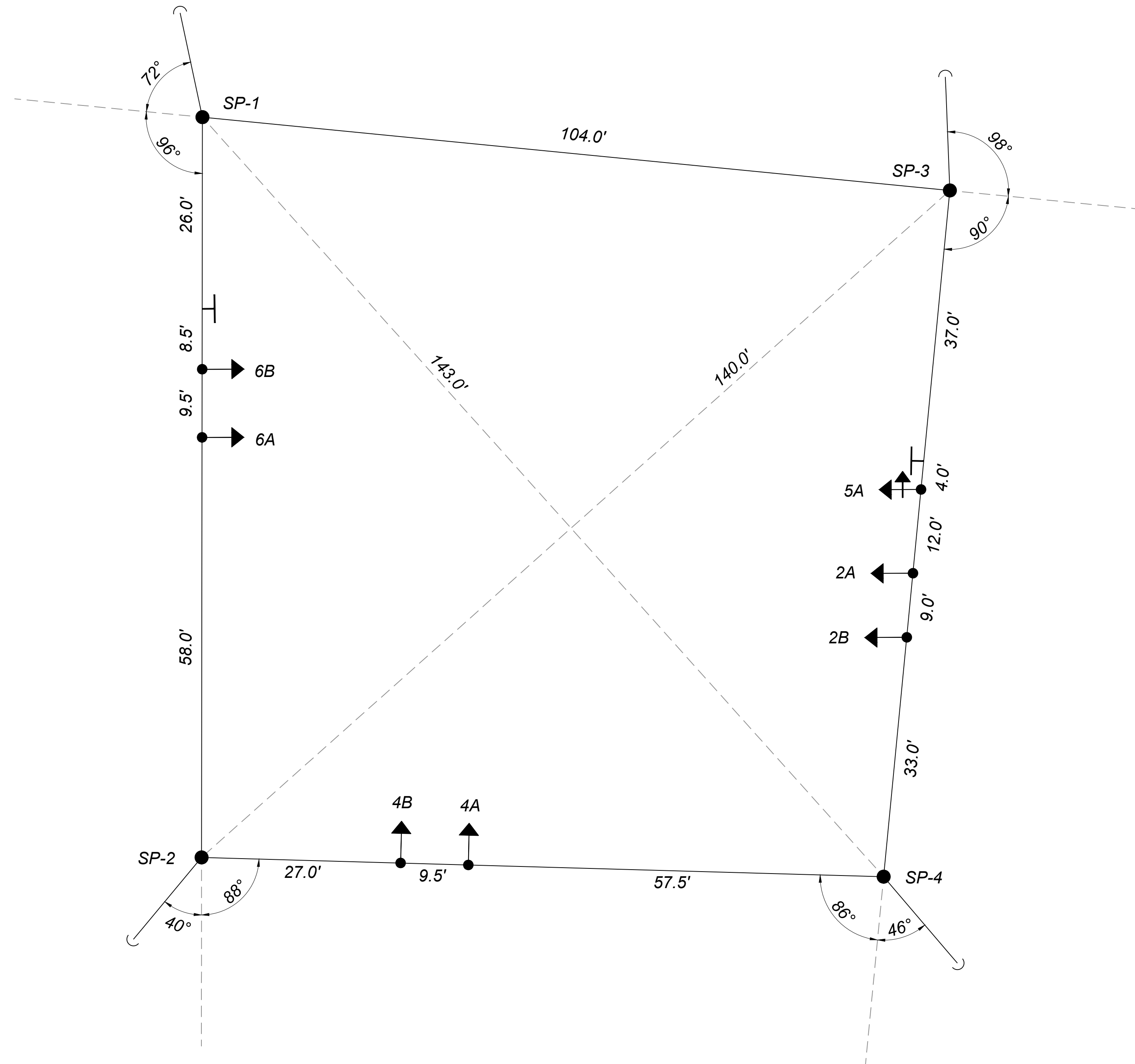


R3-5R
30" x 36"
B

LEGEND	
•	WOOD SIGNAL SUPPORT POLE
→	3-SECTION VEHICLE SIGNAL HEAD
↔	5-SECTION VEHICLE SIGNAL HEAD
— —	OVERHEAD SIGN
☒	CONTROLLER CABINET AND WORK PAD (332L)
□	PULL BOX
—	CONDUIT
⊙	DILEMMA ZONE RADAR DETECTION UNIT
⊙	STOP BAR RADAR DETECTION UNIT
▨	DETECTION ZONE

BEL-149-23.44

T7
T13



SPAN WIRE PLAN VIEW DETAIL

WOOD POLE TABLE

REFERENCE SHEET NO.	STATION & OFFSET	POLE NO.	WOOD POLE CLASS	POLE HEIGHT (FT.)	FOUNDATION ELEVATION (FT.)	SPAN WIRE ATTACHMENT HEIGHT (FT.)
T7	STA. 121+11.75, 53.99' LT	SP-1	H2	35.0	1190.95	25.62
T7	STA. 121+12.96, 49.92' RT	SP-2	H2	35.0	1192.55	25.66
T7	STA. 122+16.82, 45.03' LT	SP-3	H2	40.0	1190.23	27.07
T7	STA. 122+08.74, 51.41' RT	SP-4	H2	40.0	1193.15	27.16

CALCULATED
JMP
CHECKED
REM

TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

BEL-149-23.44

T8
T13

SIGNAL TIMING CHART

INTERSECTION: S.R. 149 & RECO DRIVE MAINTAINING AGENCY: ODOT DISTRICT 11									
START UP		DUAL ENTRY:	YES	PHASES:		2 + 6			
START IN:		ALL RED		RING 1		RING 2			
TIME FOR FLASH, ALL RED:		5 SEC.		OVERLAP		A	B	C	D
FIRST PHASE(S):		Ø2 + Ø6		PHASES		-	-	-	-
COLOR DISPLAYED:		GREEN							
INTERVAL OR FEATURE	CONTROLLER MOVEMENT NO.								
INTERSECTION MOVEMENT (PHASE)	1	2	3	4	5	6	7	8	
DIRECTION	-	NB	-	EB	NBLT	SB	-	-	
MINIMUM GREEN (INITIAL) (SEC.)	-	20	-	10	7	20	-	-	
ADDED INITIAL *(SEC./ACTUATION)	-	-	-	-	-	-	-	-	
MAXIMUM INITIAL *(SEC.)	-	-	-	-	-	-	-	-	
PASSAGE TIME (PRESET GAP) (SEC.)	-	2.0	-	2.0	2.0	2.0	-	-	
TIME BEFORE REDUCTION *(SEC.)	-	-	-	-	-	-	-	-	
MINIMUM GAP *(SEC.)	-	-	-	-	-	-	-	-	
TIME TO REDUCE *(SEC.)	-	-	-	-	-	-	-	-	
MAXIMUM GREEN I (SEC.)	-	34	-	14	8	21	-	-	
MAXIMUM GREEN II (SEC.)	-	-	-	-	-	-	-	-	
YELLOW CHANGE (SEC.)	-	5.0	-	4.5	4.0	5.0	-	-	
ALL RED CLEARANCE (SEC.)	-	1.0	-	1.0	1.5	1.0	-	-	
DELAYED GREEN (LPI) # (SEC.)	-	-	-	-	-	-	-	-	
WALK (SEC.)	-	-	-	-	-	-	-	-	
PEDESTRIAN CLEARANCE (SEC.)	-	-	-	-	-	-	-	-	
RECALL	MAXIMUM (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF
	MINIMUM (ON/OFF)	-	ON	-	OFF	-	ON	-	OFF
	PEDESTRIAN (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF
MEMORY (ON/OFF)	-	OFF	-	OFF	-	OFF	-	OFF	

*VOLUME DENSITY CONTROLS

NOTES:

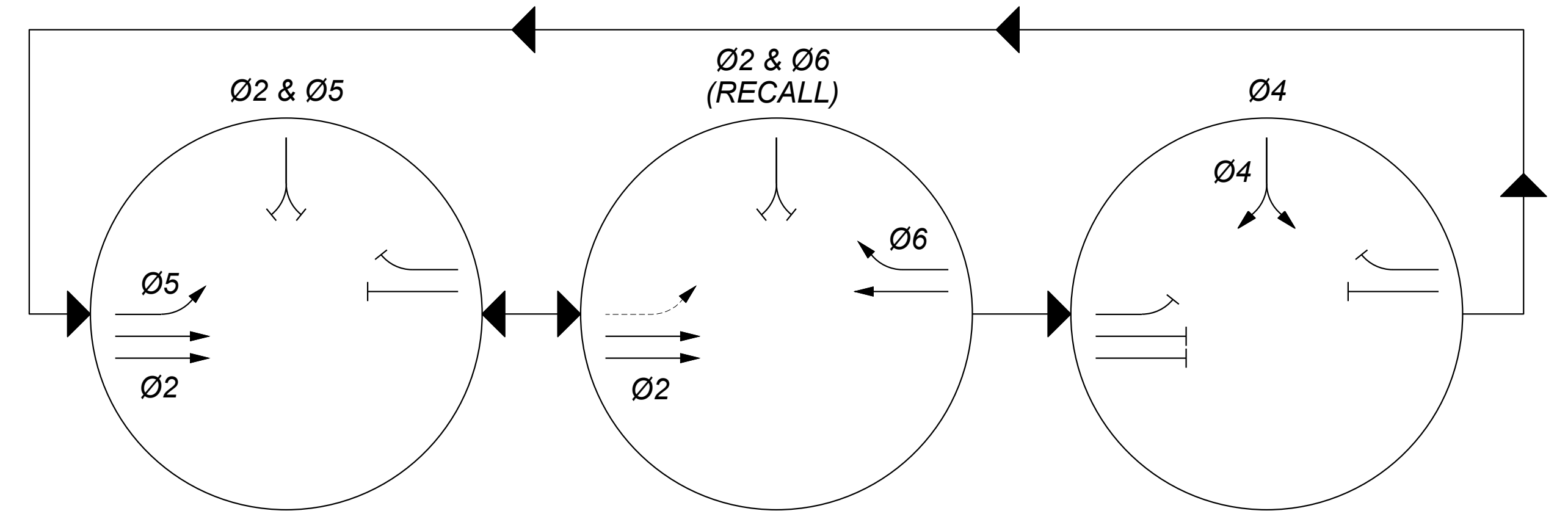
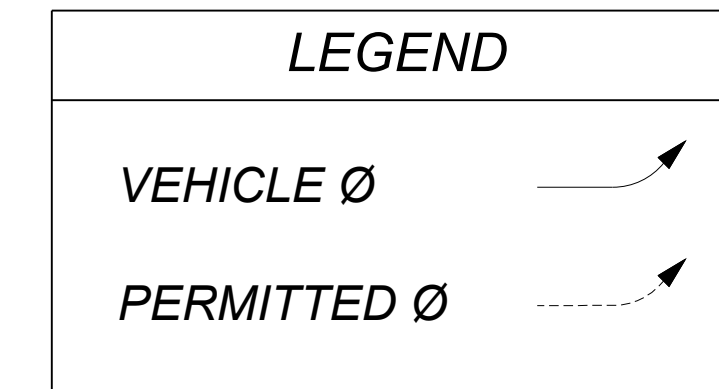
- ALL MOVEMENTS SHALL BE ACTUATED. THE PRIMARY THRU MOVEMENT SHOULD HAVE MIN RECALL ACTIVE TO REST IN GREEN.

-RADAR DETECTION UNITS FOR DILEMMA ZONE DETECTION SHALL PLACE A CONSTANT CALL TO THE CONTROLLER WHEN VEHICLE TRAVEL TIMES TO THE STOP BAR ARE BETWEEN 2.5 AND 6.0 SECONDS. SPEED TRIGGER SHALL BE SET FOR VEHICLES TRAVELING 35 MPH AND GREATER.

-RADAR SHALL HAVE QUEUE DETECTION CONFIGURED AND A ZONE PLACED AT 100-200 FEET FROM THE STOP BAR FOR SLOW MOVING VEHICLE EXTENSIONS. SPEED TRIGGER SHALL BE SET AT 1-35 MPH.

- ALL DETECTOR DELAYS SHALL BE PLACED IN THE CONTROLLER.

PHASING DIAGRAM



RADAR DETECTION CHART

DETECTION ZONE	MOVEMENT	PULSE OR PRESENCE	ASSOCIATED PHASE	DELAY PROGRAMMED IN CONTROLLER (SEC.)	EXTENSION PROGRAMMED IN CONTROLLER (SEC.)	DETECTOR NO.	PURPOSE	DETECTION ZONE LENGTH (FT)
RDZ5A	NB LT	PRESENCE	Ø5	5.0	2.0	D2A	STOP BAR	40
RDZ2A	NB	PULSE	Ø2	0.0	2.0	D2B	DILEMMA ZONE	850
RDZ4A	EB LR	PRESENCE	Ø4	10.0	2.0	D4A	STOP BAR	40
RDZ6A	SB	PULSE	Ø6	0.0	2.0	D6A	DILEMMA ZONE	850

NOTE: ALL DETECTION ZONES SHALL BE CENTERED IN THEIR RESPECTIVE LANES UNLESS OTHERWISE SHOWN.

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JMP
CHECKED
REM

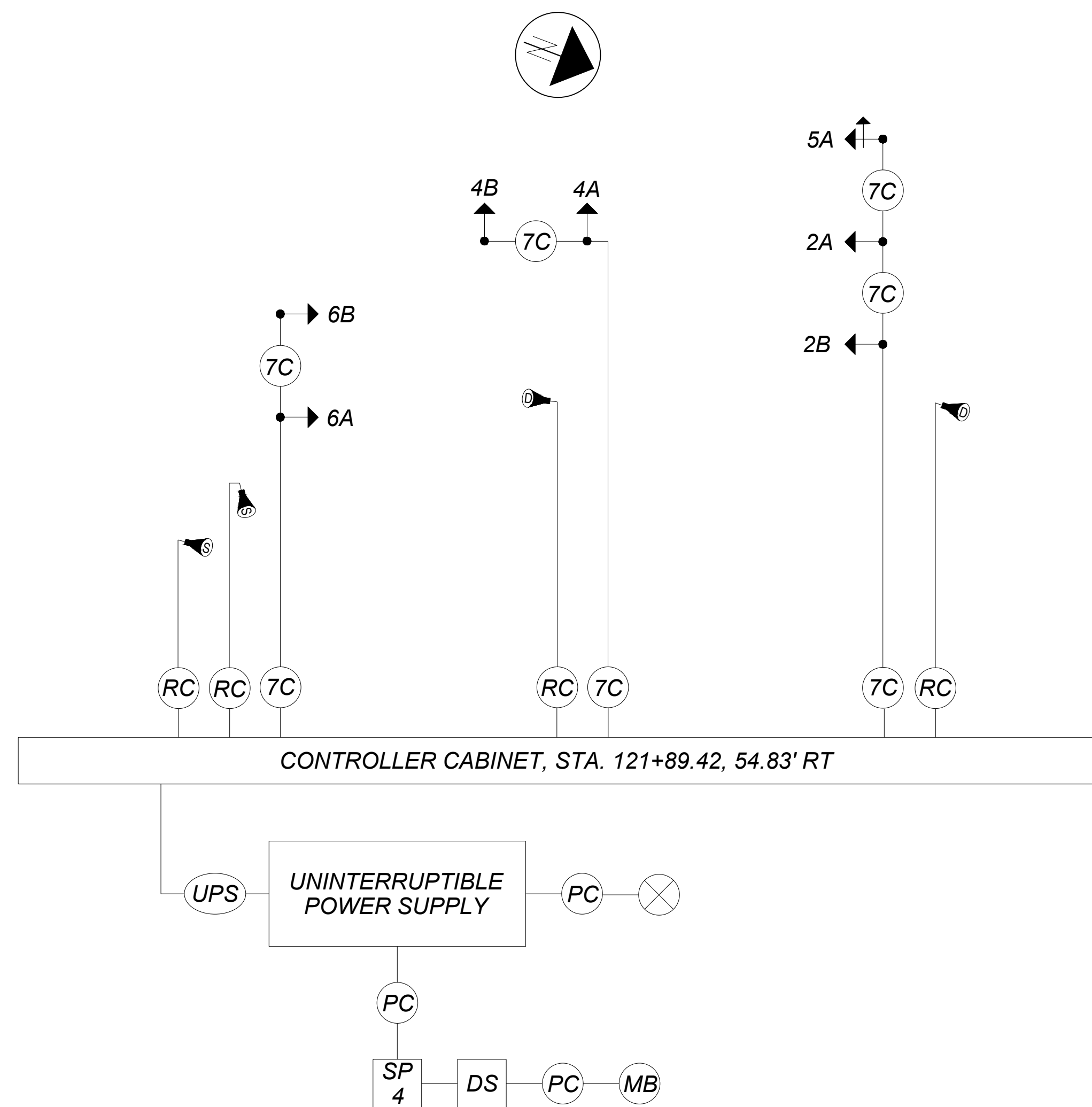
TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

BEL-149-23.44

T9
T13

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WIRING DIAGRAM



WIRING DIAGRAM LEGEND

	5-SECTION VEHICULAR SIGNAL HEAD, 1-WAY		POWER SOURCE
	3-SECTION VEHICULAR SIGNAL HEAD, 1-WAY		POWER CABLE, 2 CONDUCTOR, NO. 6 AWG
	STOP BAR RADAR DETECTION UNIT		METER BASE
	DILEMMA ZONE RADAR DETECTION UNIT		SIGNAL DISCONNECT SWITCH
	SIGNAL CABLE, 7 CONDUCTOR, NO. 14 AWG		UNINTERRUPTIBLE POWER SUPPLY CABLE
	RADAR DETECTION CABLE		

TRAFFIC SIGNAL SUBSUMMARY

ITEM	DESCRIPTION	QUANTITY	UNIT
625	CONDUIT, 2", 725.04	FT	20
625	CONDUIT, 3", 725.04	FT	20
625	TRENCH	FT	20
625	PULL BOX, 725.08, 24"	EACH	1
625	GROUND ROD	EACH	5
625	UNDERGROUND WARNING/MARKING TAPE	FT	20
625	ARC FLASH CALCULATIONS AND LABEL	EACH	1
630	SIGN HANGER ASSEMBLY, SPAN WIRE	EACH	2
630	SIGN, FLAT SHEET	SF	15
632	VEHICULAR SIGNAL HEAD, (LED), 3-SECTION, 12" LENS, 1-WAY, POLYCARBONATE	EACH	6
632	VEHICULAR SIGNAL HEAD, (LED), 5-SECTION, 12" LENS, 1-WAY, POLYCARBONATE	EACH	1
632	COVERING OF VEHICULAR SIGNAL HEAD	EACH	7
632	MESSENGER WIRE, 7 STRAND, 3/8" DIAMETER WITH ACCESSORIES	FT	435
632	TETHER WIRE, WITH ACCESSORIES	FT	435
632	SIGNAL CABLE, 7 CONDUCTOR, NO. 14 AWG	FT	518
632	WOOD POLE	EACH	4
632	DOWN GUY	EACH	4
632	POWER CABLE, 2 CONDUCTOR, NO. 6 AWG	FT	40
632	POWER SERVICE, AS PER PLAN	EACH	1
632	CONDUIT RISER, 2" DIAMETER	EACH	2
633	CABINET, TYPE 332L, AS PER PLAN	EACH	1
633	CABINET FOUNDATION	EACH	1
633	CONTROLLER WORK PAD	EACH	1
633	UNINTERRUPTIBLE POWER SUPPLY (UPS), 1000 WATT, AS PER PLAN	EACH	1
633	CONTROLLER ITEM, MISC.: TIMING AND COORDINATION	EACH	1
633	COMMUNICATIONS, AS PER PLAN	EACH	1
809	ADVANCED RADAR DETECTION, AS PER PLAN	EACH	2
809	STOP LINE RADAR DETECTION, AS PER PLAN	EACH	2
809	ATC CONTROLLER, AS PER PLAN	EACH	1

FIELD WIRING HOOK-UP CHART

SIGNAL HEAD	INDICATION	FIELD TERMINAL	FLASH
5A (NB LT)	R	Ø2 R	R
	Y	Ø2 Y	
	G	Ø2 G	
	<--Y-->	Ø5 Y	
2A, 2B (NB)	R	Ø2 R	R
	Y	Ø2 Y	
	G	Ø2 G	
4A, 4B (EB)	R	Ø4 R	R
	Y	Ø4 Y	
	G	Ø4 G	
6A, 6B (SB)	R	Ø6 R	R
	Y	Ø6 Y	
	G	Ø6 G	

LS = LOAD SWITCH

TRAFFIC SIGNAL PLAN DETAILS
S.R. 149

BEL-149-23.44

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COORDINATION TIME CHART

"INTERSECTION 1" - S.R. 149 & RECO DRIVE										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	NBLT	SB	-	-		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	40	-	20	13.4	26.6	-	-	55	-
2	-	52.6	-	17.4	13.4	39.2	-	-	64	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

"INTERSECTION 3" - I-70 WESTBOUND RAMPS & S.R. 149 (MASTER)										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	WB	-	SB	-	WB		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	37.5	-	22.5	-	37.5	-	-	0	-
2	-	43	-	27	-	43	-	-	0	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

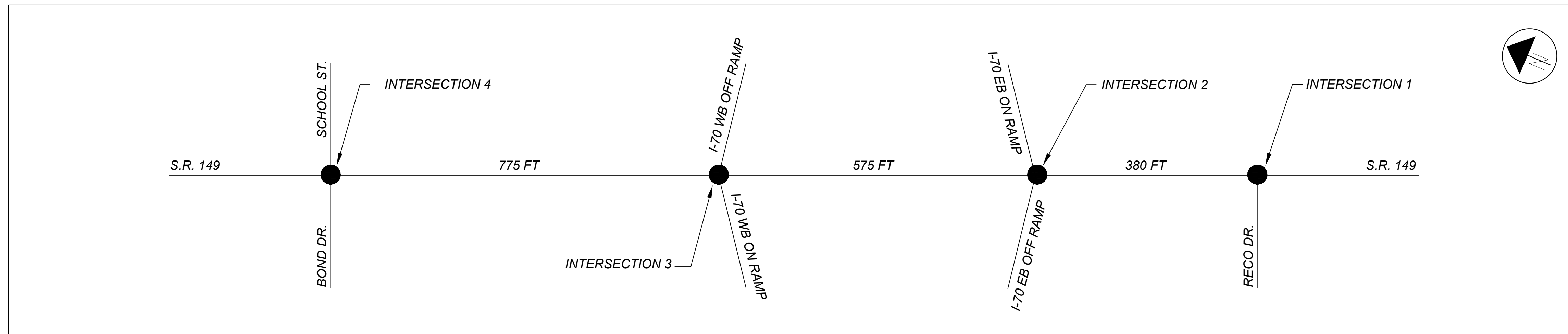
"INTERSECTION 2" - I-70 EASTBOUND RAMPS & S.R. 149										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	-	SB	-	-		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	42	-	18	-	42	-	-	52	-
2	-	47.5	-	22.5	-	47.5	-	-	62	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

"INTERSECTION 4" - S.R. 149 & BOND DRIVE/SCHOOL STREET										
PHASE	1	2	3	4	5	6	7	8	OFFSET 1 (SEC)	OFFSET 2 (SEC)
DIRECTION	-	NB	-	EB	-	SB	-	WB		
PLAN NO. OR C/S/O	SPLITS (G+Y+AR) IN SECONDS									
-	-	-	-	-	-	-	-	-	-	-
1	-	37	-	23	-	37	-	23	12	-
2	-	42	-	28	-	42	-	28	5	-
-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	-	-	-	-	-

COORDINATION TIMING PLANS

DAY(S) OF WEEK	PLAN NAME	HOURS	PLAN NO. OR CYCLE/SPLIT/OFFSET	CYCLE LENGTH (SEC)
-	-	-	-	-
SAT-SUN	FREE	0000 - 2400	FREE	FREE
-	-	-	-	-
MON-FRI	FREE	0000 - 0700	FREE	FREE
MON-FRI	AM PEAK	0700 - 0930	1	60
MON-FRI	FREE	0930 - 1430	FREE	FREE
MON-FRI	PM PEAK	1430 - 1800	2	70
MON-FRI	FREE	1800 - 2400	FREE	FREE
-	-	-	-	-

COORDINATION LAYOUT



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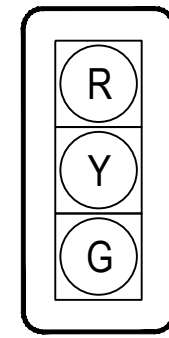
CALCULATED
JMP
CHECKED
REM

TRAFFIC SIGNAL COORDINATION PLAN
S.R. 149

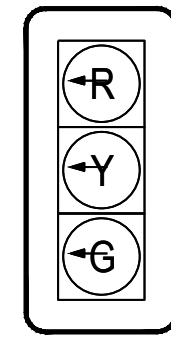
BEL-149-23.44

T11
T13

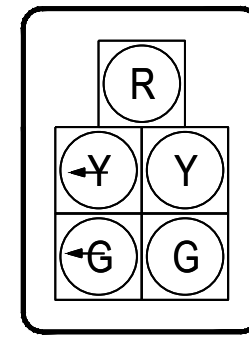
**SIGNAL HEADS, 12", LED
WITH BACK PLATES**



SIGNALS
2A, 2B, 2C,
4C, 6A, 6C

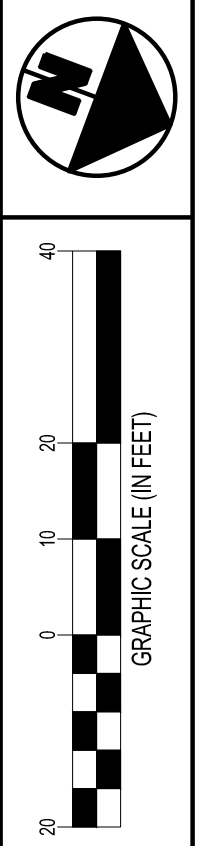


SIGNALS
4A



SIGNALS
4B, 6B

TRAFFIC SIGNAL SUBSUMMARY			
ITEM	QUANTITY	UNIT	
614	1	LS	MAINTAINING TRAFFIC
632	1	EACH	VEHICULAR SIGNAL HEAD, (LED), 5-SECTION, 12" LENS, 1-WAY, POLYCARBONATE
809	1	EACH	STOP LINE RADAR DETECTION, AS PER PLAN



CALCULATED
TMC
CHECKED
REM

EXISTING 3-SECTION SIGNAL HEAD
TO BE REPLACED WITH 5-SECTION
SIGNAL HEAD.

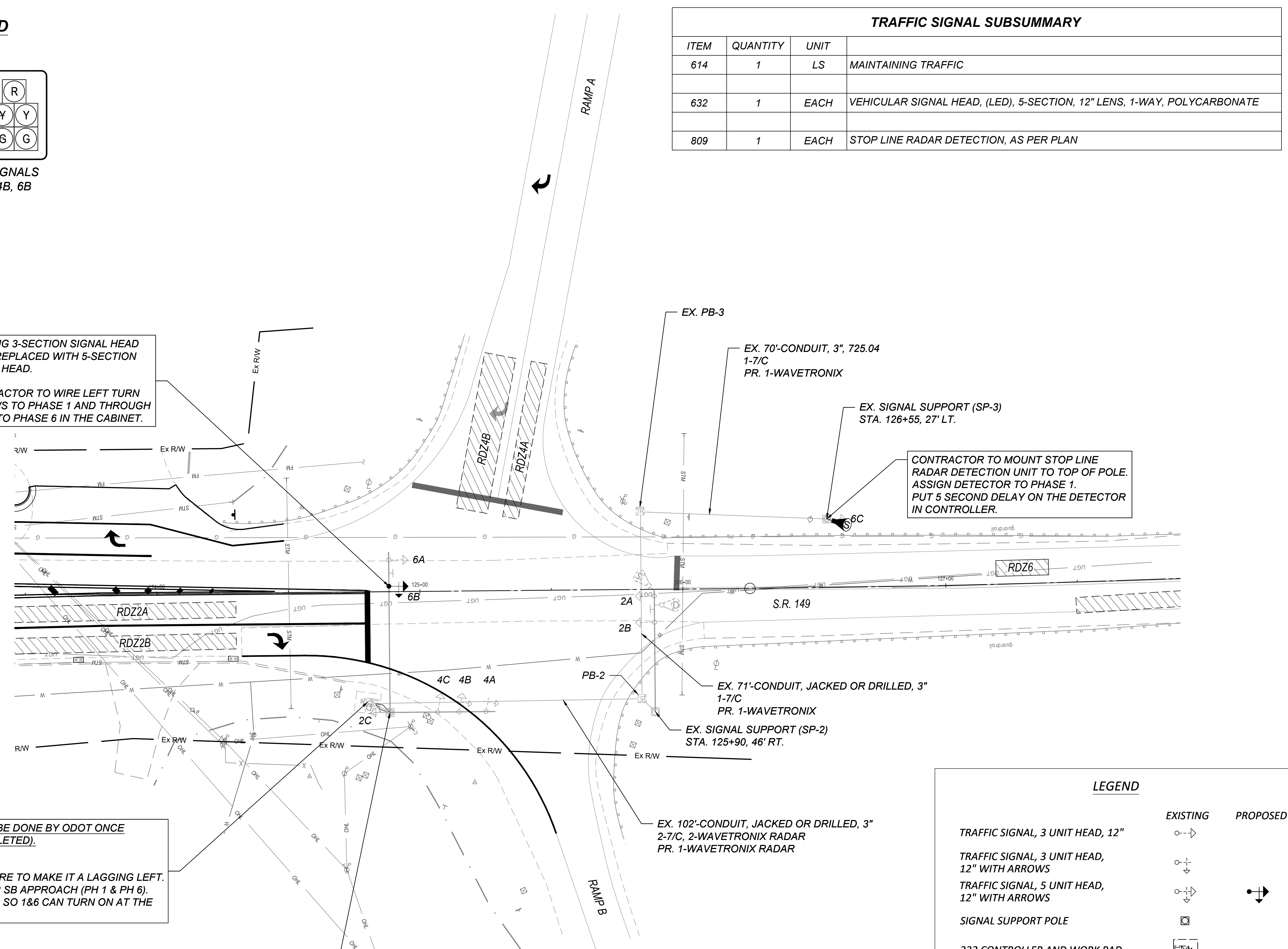
CONTRACTOR TO WIRE LEFT TURN
ARROWS TO PHASE 1 AND THROUGH
BALLS TO PHASE 6 IN THE CABINET.

CONTRACTOR TO MOUNT STOP LINE
RADAR DETECTION UNIT TO TOP OF POLE.
ASSIGN DETECTOR TO PHASE 1.
PUT 5 SECOND DELAY ON THE DETECTOR
IN CONTROLLER.

CONTROLLER WORK (TO BE DONE BY ODOT ONCE
CONSTRUCTION IS COMPLETED).

- ACTIVATE PHASE 1.
- CHANGE RING STRUCTURE TO MAKE IT A LAGGING LEFT.
- ENABLE PREEMPT 3 FOR SB APPROACH (PH 1 & PH 6).
- SOLDER MONITOR CARD SO 1&6 CAN TURN ON AT THE SAME TIME.

PULL BOX DATA			
NO.	STATION	OFFSET	SIZE
PB-1	124+82.8	42' RT.	24"
PB-2	125+85	41' RT.	18"
PB-3	125+85	30' LT.	18"



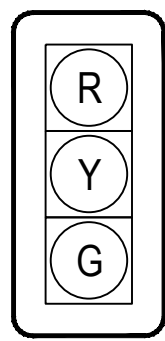
	EXISTING	PROPOSED
TRAFFIC SIGNAL, 3 UNIT HEAD, 12"		
TRAFFIC SIGNAL, 3 UNIT HEAD, 12" WITH ARROWS		
TRAFFIC SIGNAL, 5 UNIT HEAD, 12" WITH ARROWS		
SIGNAL SUPPORT POLE		
332 CONTROLLER AND WORK PAD		
TRAFFIC PULL BOX		
DILEMMA ZONE RADAR DETECTION UNIT		
STOP LINE RADAR DETECTION UNIT		
DETECTION ZONE		

TRAFFIC SIGNAL PLAN
S.R. 149 & EASTBOUND I.R. 70 RAMPS

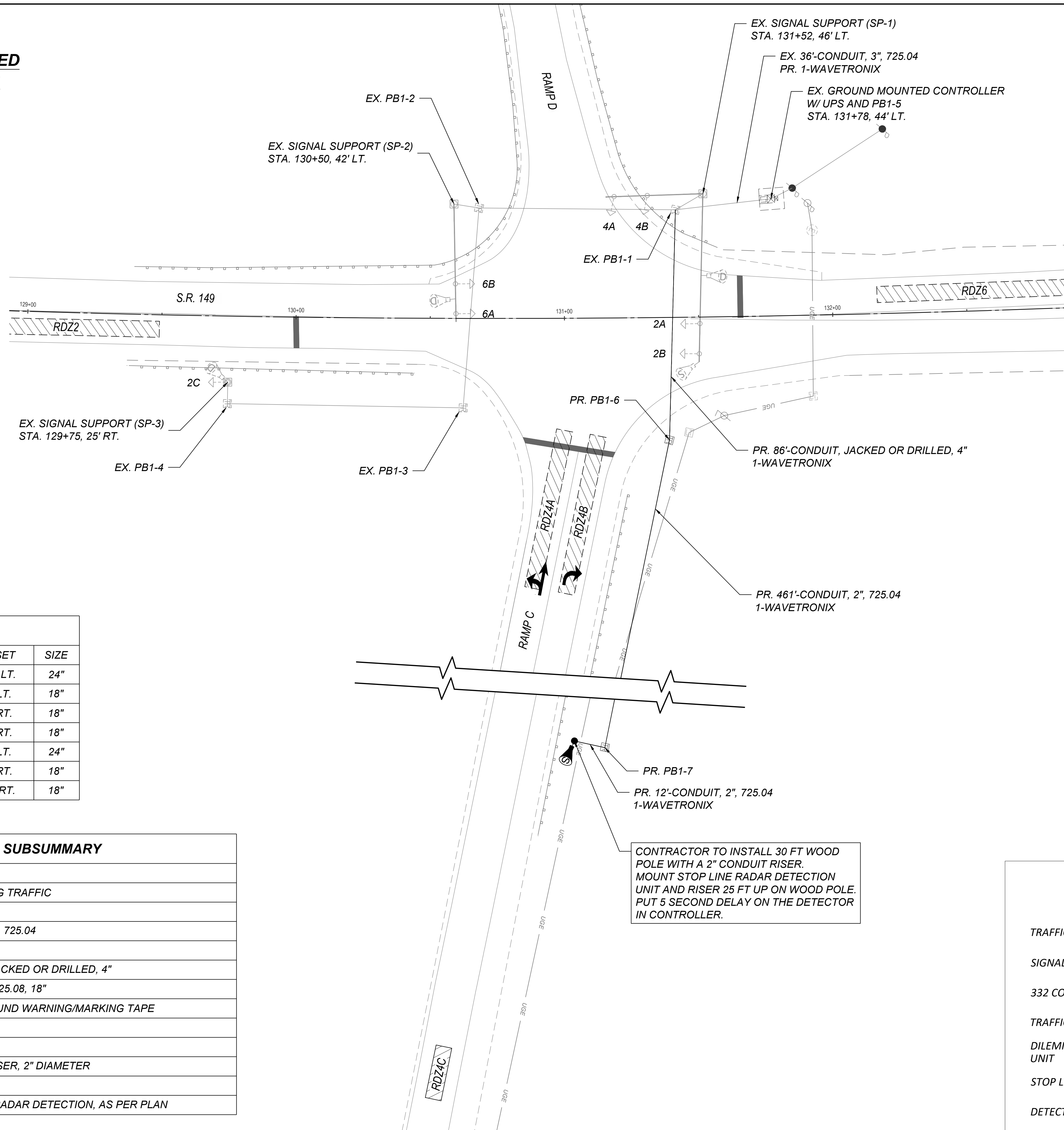
BEL-149-23.44

T12
T13

**SIGNAL HEADS, 12", LED
WITH BACK PLATES**



SIGNALS
2A, 2B, 2C,
4A, 4B, 6A, 6B



PULL BOX DATA

	NO.	STATION	OFFSET	SIZE
EX.	PB1-1	131+42	40.5' LT.	24"
EX.	PB1-2	130+68	41' LT.	18"
EX.	PB1-3	130+68	31' RT.	18"
EX.	PB1-4	129+75	30' RT.	18"
EX.	PB1-5	131+78	44' LT.	24"
PR.	PB1-6	131+39	46' RT.	18"
PR.	PB1-7	130+49	497' RT.	18"

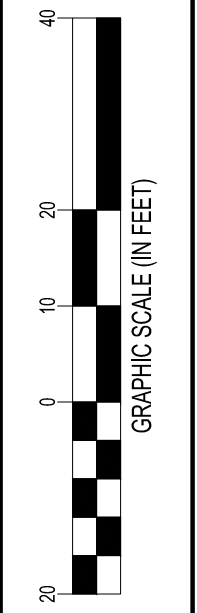
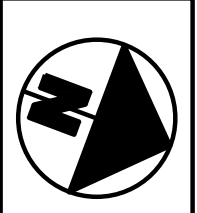
TRAFFIC SIGNAL SUBSUMMARY

ITEM	QUANTITY	UNIT	
614	1	LS	MAINTAINING TRAFFIC
625	473	FT	CONDUIT, 2", 725.04
625	473	FT	TRENCH
625	86	FT	CONDUIT, JACKED OR DRILLED, 4"
625	2	EACH	PULL BOX, 725.08, 18"
625	473	FT	UNDERGROUND WARNING/MARKING TAPE
632	1	EACH	WOOD POLE
632	1	EACH	CONDUIT RISER, 2" DIAMETER
809	1	EACH	STOP LINE RADAR DETECTION, AS PER PLAN

CONTRACTOR TO INSTALL 30 FT WOOD POLE WITH A 2" CONDUIT RISER. MOUNT STOP LINE RADAR DETECTION UNIT AND RISER 25 FT UP ON WOOD POLE. PUT 5 SECOND DELAY ON THE DETECTOR IN CONTROLLER.

LEGEND

	EXISTING	PROPOSED
TRAFFIC SIGNAL, 3 UNIT HEAD, 12"		
SIGNAL SUPPORT POLE		
332 CONTROLLER AND WORK PAD		
TRAFFIC PULL BOX		
DILEMMA ZONE RADAR DETECTION UNIT		
STOP LINE RADAR DETECTION UNIT		
DETECTION ZONE		



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TMC
CHECKED
REM

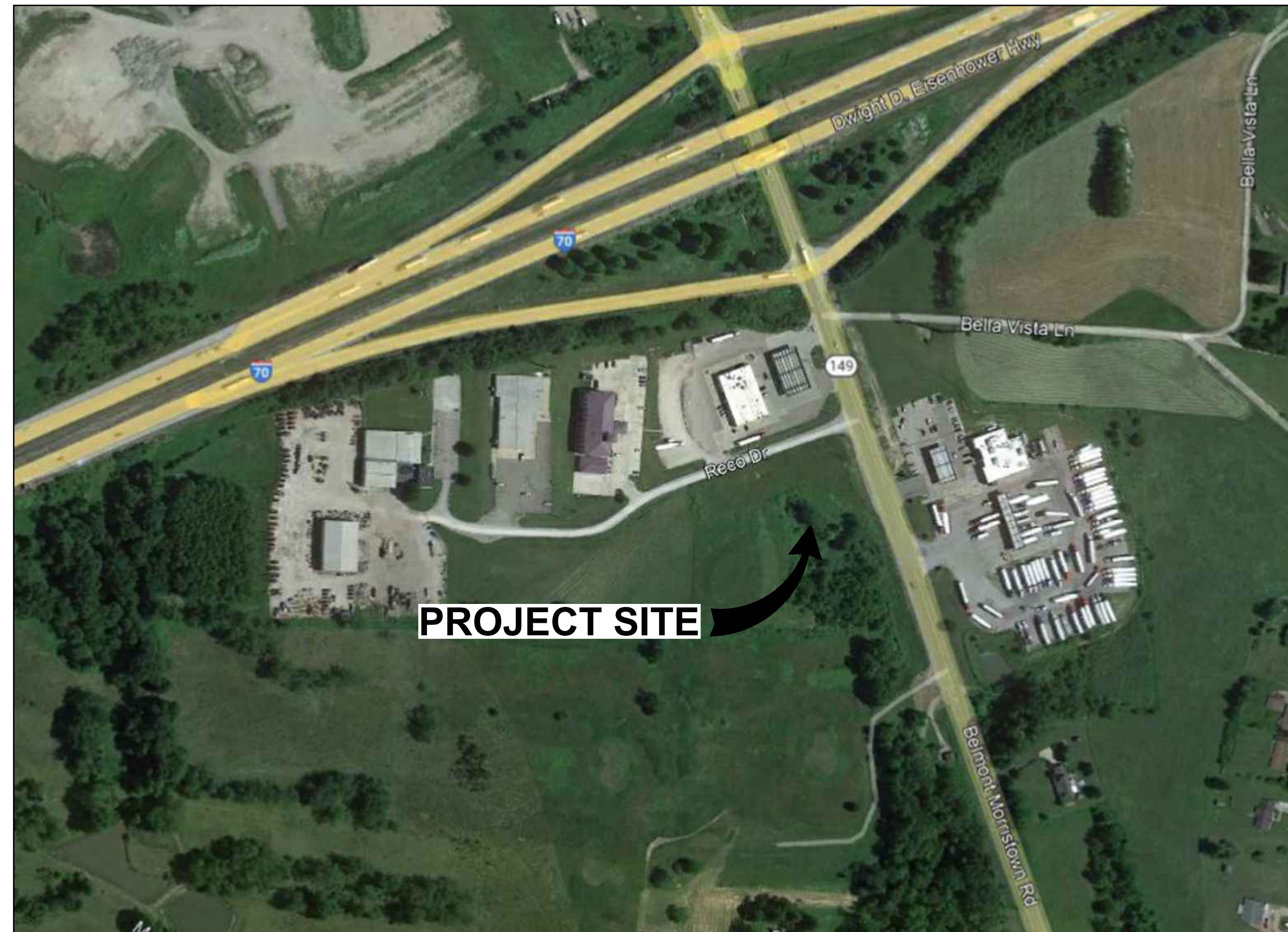
**TRAFFIC SIGNAL PLAN
S.R. 149 & WESTBOUND I.R. 70 RAMPS**

BEL-149-23.44

T13
T13

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PROPOSED REINFORCED SOIL SLOPE (RSS) AND EMBANKMENT SLOPES FOR LOVE'S TRAVEL STOP - S.R. 149 ROADWAY SLOPES ST. CLAIRSVILLE, OHIO



NOTE: SITE IMAGE PROVIDED BY GOOGLE EARTH, PRO, 2023.

VICINITY MAP

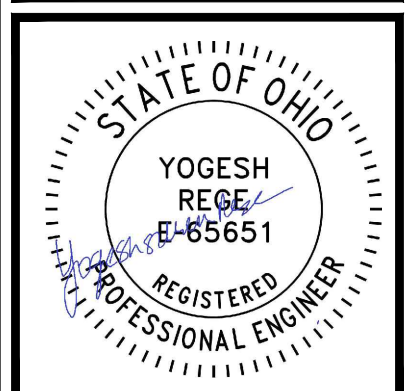
PREPARED FOR:



SHEET INDEX	
SHEET NUMBER	SHEET DESCRIPTION
GR-1	COVER SHEET
GR-2	TECHNICAL SCOPE OF WORK
GR-3	PROPOSED SLOPE EXTENTS AND SITE PLAN
GR-4	TYPICAL SLOPE SECTIONS
GR-5	TYPICAL SLOPE SECTIONS
GR-6	SLOPE CONSTRUCTION DETAILS

REV	DATE	BY	DESCRIPTION
10	06/12/2024	MWF	CC010

COVER SHEET
 LOVE'S TRAVEL STOP - S.R. 149 ROADWAY SLOPES
 66320 BELMONT-MORRISTOWN ROAD
 ST. CLAIRSVILLE OHIO

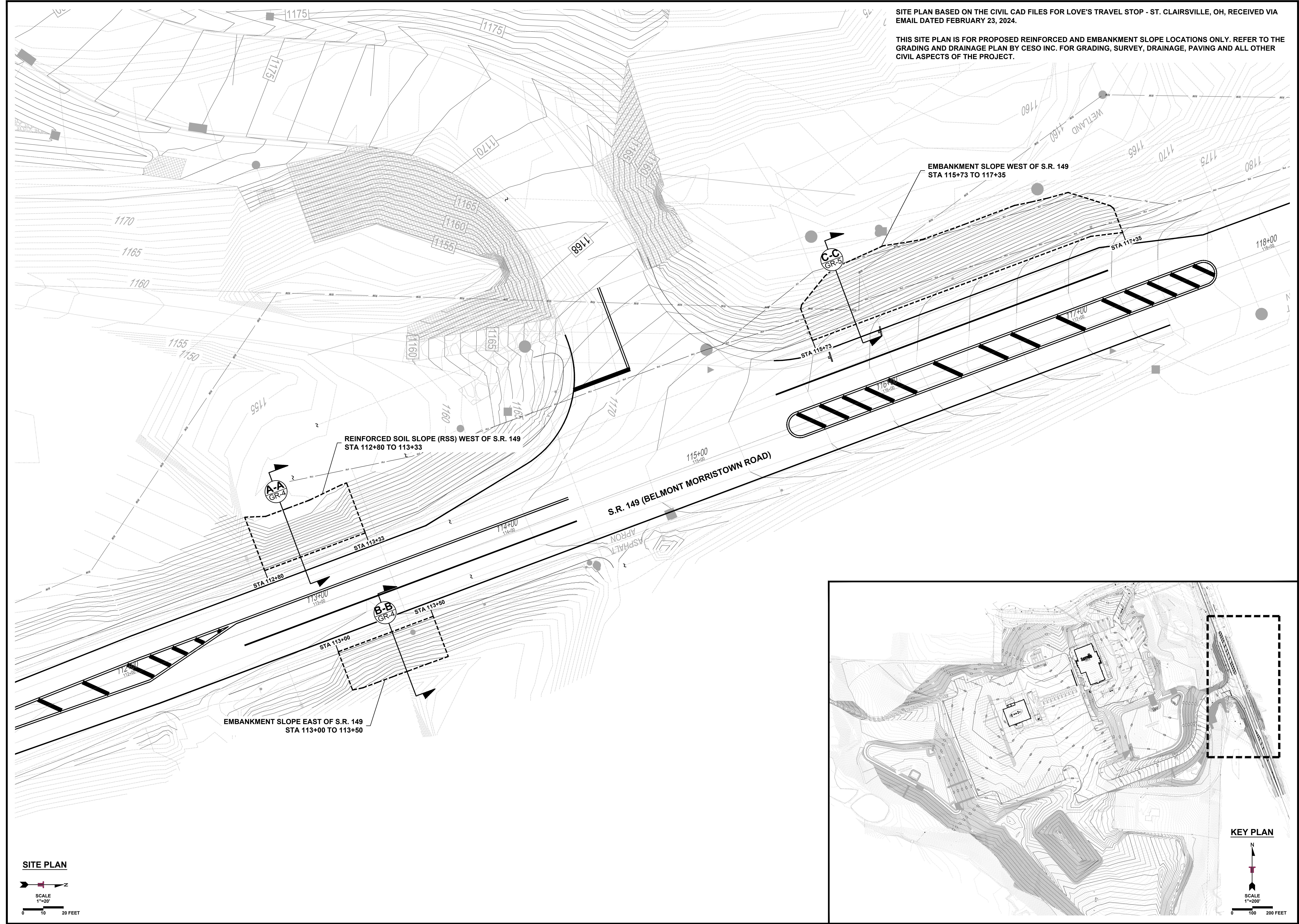


GR-1	
DESIGNED BY:	JP
DRAWN BY:	MWF
APPRD. BY:	TMK
SCALE:	AS SHOWN
DATE:	9/06/24
JOB NO.:	65235270
ACAD NO.:	1 GR-1
SHEET NO.:	1 OF 6

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SITE PLAN BASED ON THE CIVIL CAD FILES FOR LOVE'S TRAVEL STOP - ST. CLAIRSVILLE, OH, RECEIVED VIA EMAIL DATED FEBRUARY 23, 2024.

THIS SITE PLAN IS FOR PROPOSED REINFORCED AND EMBANKMENT SLOPE LOCATIONS ONLY. REFER TO THE GRADING AND DRAINAGE PLAN BY CESO INC. FOR GRADING, SURVEY, DRAINAGE, PAVING AND ALL OTHER CIVIL ASPECTS OF THE PROJECT.



SITE PLAN

SCALE 1"=20'

0 10 20 FEET

KEY PLAN

SCALE 1"=200'

0 100 200 FEET

REV.	DATE	BY	DESCRIPTION
10	06-12-2024	MMWF	GG010

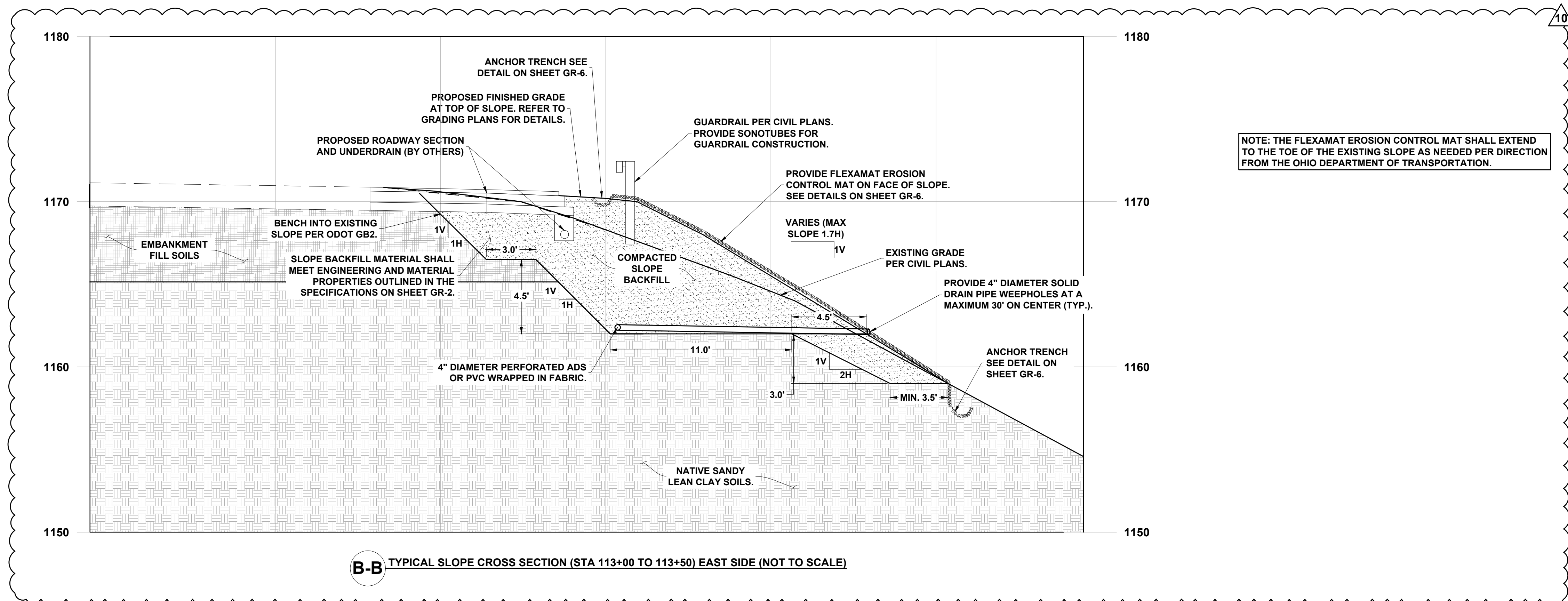
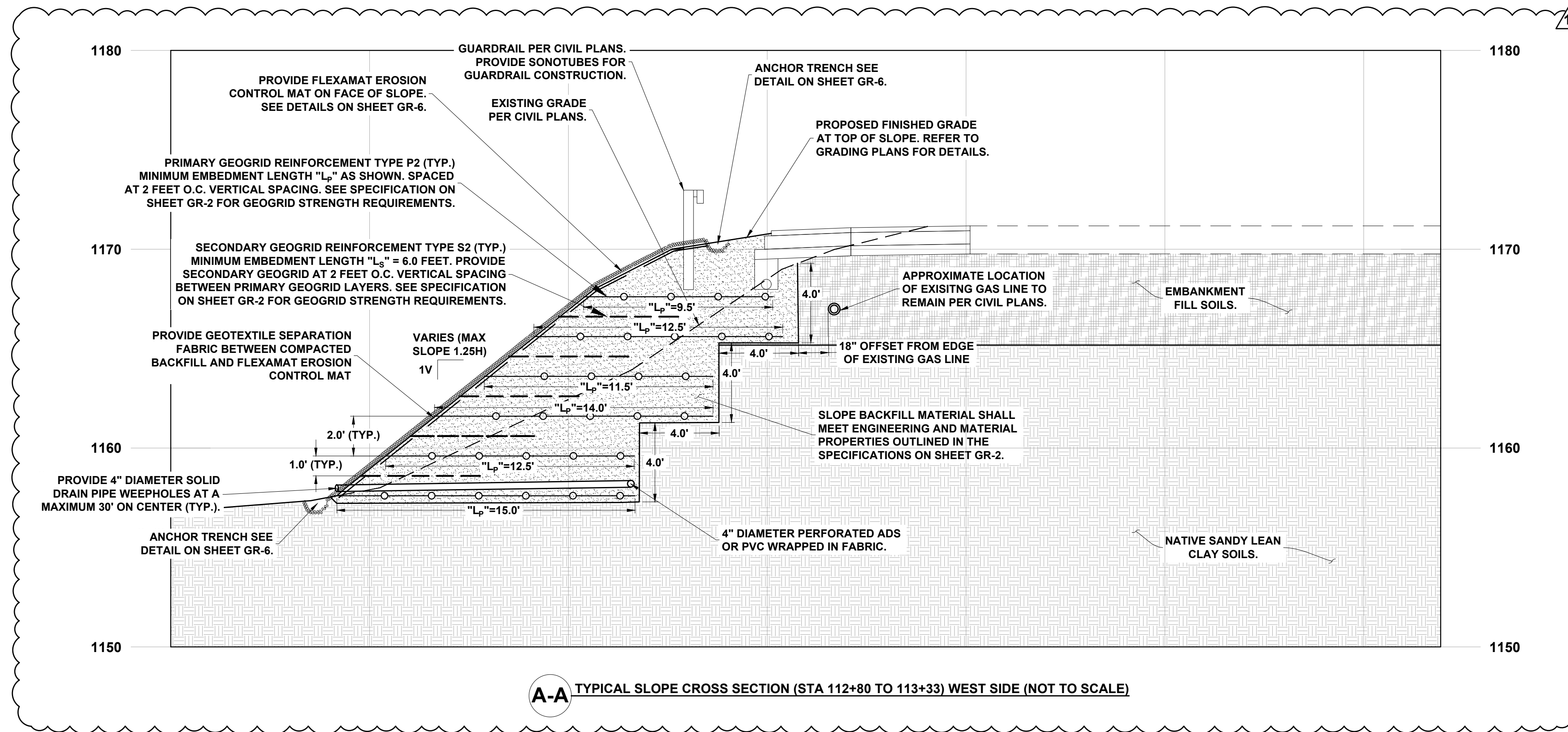
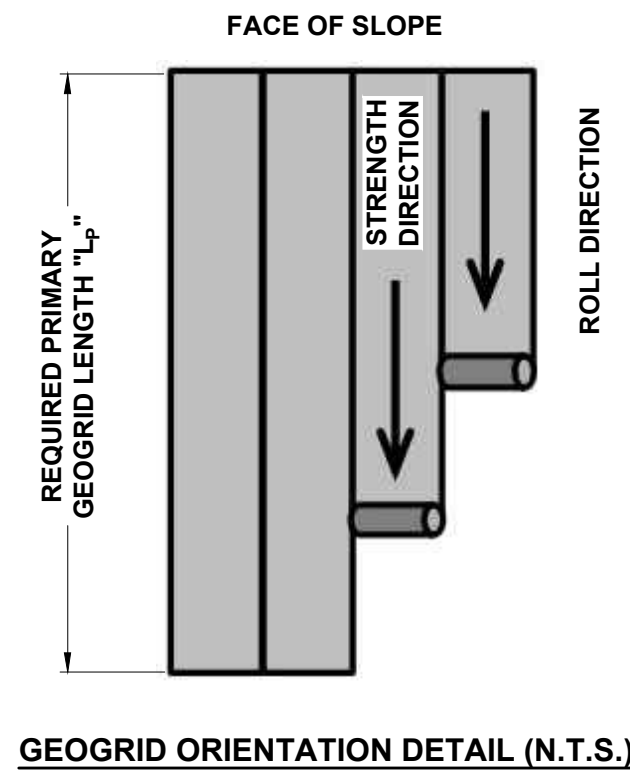
PROPOSED SLOPE EXTENTS AND SITE PLAN
LOVE'S TRAVEL STOP - S.R. 149 ROADWAY SLOPES
 66320 BELMONT-MORRISTOWN ROAD
 ST. CLAIRSVILLE, OHIO

4685 SOUTH ASH AVENUE, SUITE 114
 PH. (603) 897-3200
 TEMPE, AZ 85282
 FAX. (603) 897-1333

STATE OF OHIO
 YOGESH
 REG. E-85651
 REGISTERED PROFESSIONAL ENGINEER

GR-3	
DESIGNED BY:	JP
DRAWN BY:	MMWF
APPVD. BY:	TMK
SCALE:	AS SHOWN
DATE:	9/06/24
JOB NO.:	65235270
ACAD NO.:	3 GR-3
SHEET NO.:	3 OF 6

N:\PROJECTS\18282\2024\PROJECT DOCUMENTS\Reports\Letter+P\PLTs to Clients\Clients\18282_18282_Slope+Site Plan.dwg LAST SAVED 9/16/2024 9:02 AM AND PLOTTED 9/16/2024 9:02 AM



NOTE: THE FLEXAMAT EROSION CONTROL MAT SHALL EXTEND TO THE TOE OF THE EXISTING SLOPE AS NEEDED PER DIRECTION FROM THE OHIO DEPARTMENT OF TRANSPORTATION.

REV	DATE	BY	DESCRIPTION
10	08-12-2024	MWF	

TYPICAL SLOPE SECTIONS

LOVE'S TRAVEL STOP - S.R. 149 ROADWAY SLOPES

66320 BELMONT-MORRISTOWN ROAD

ST. CLAIRSVILLE

OHIO

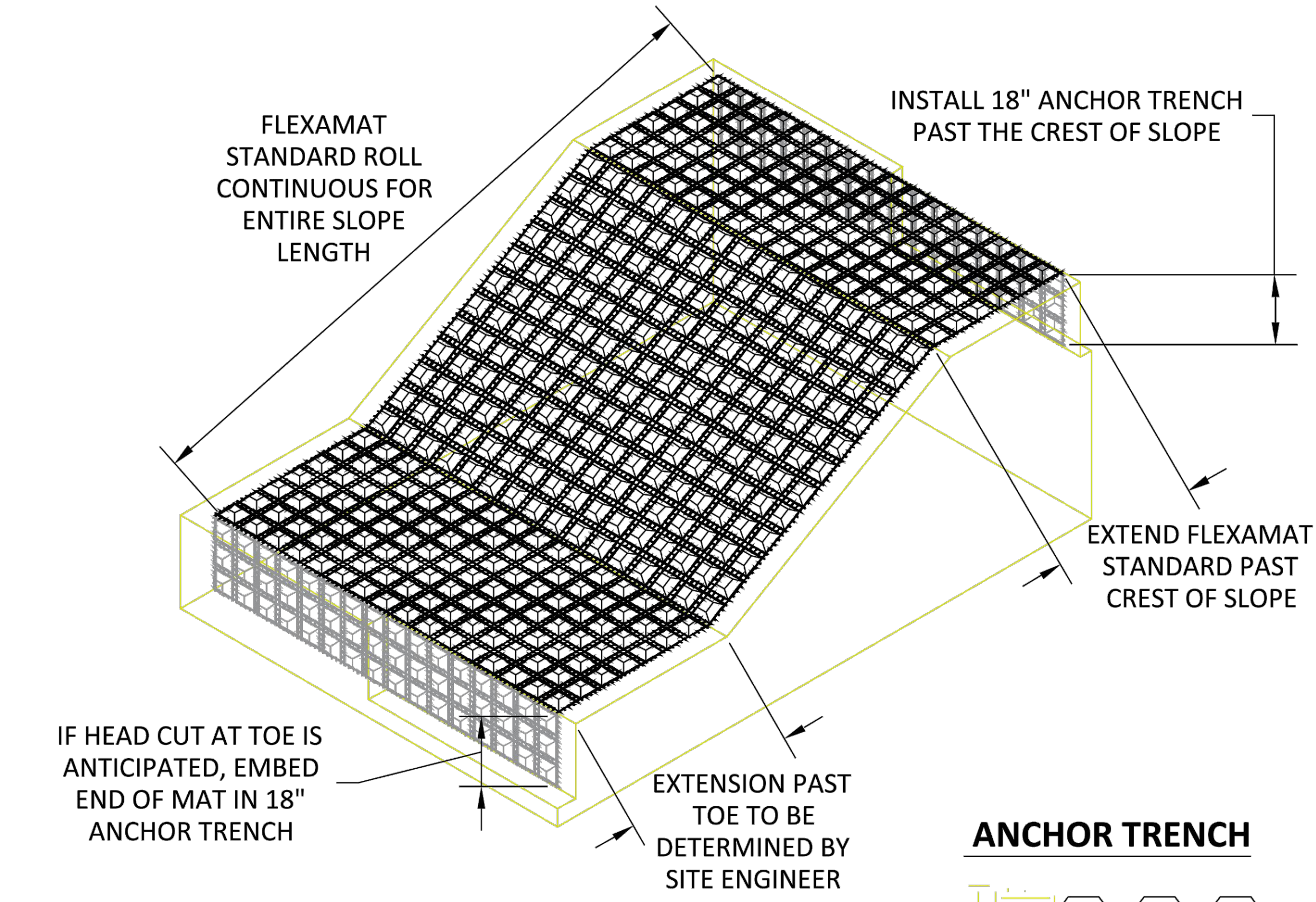
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4885 SOUTH ASH AVENUE, SUITE 114
PH: (680) 897-8200
TEMPE, AZ 85282
FAX: (680) 897-1133

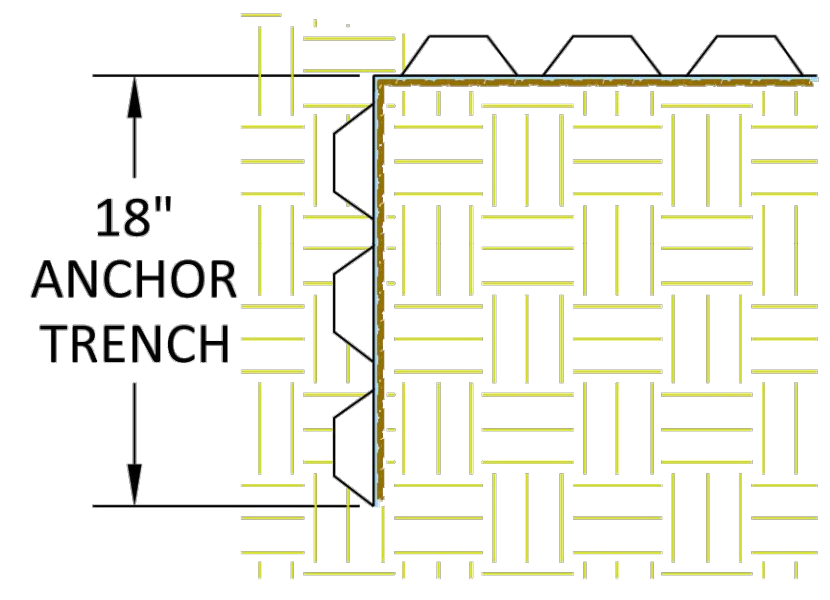
STATE OF OHIO
YOGESH REGE
E-65651
REGISTERED PROFESSIONAL ENGINEER

GR-4	
DESIGNED BY:	JP
DRAWN BY:	MWF
APP'D BY:	TMK
SCALE:	AS SHOWN
DATE:	9/06/24
JOB NO.:	65235270
ACAD NO.:	4 GR-4
SHEET NO.:	4 OF 6

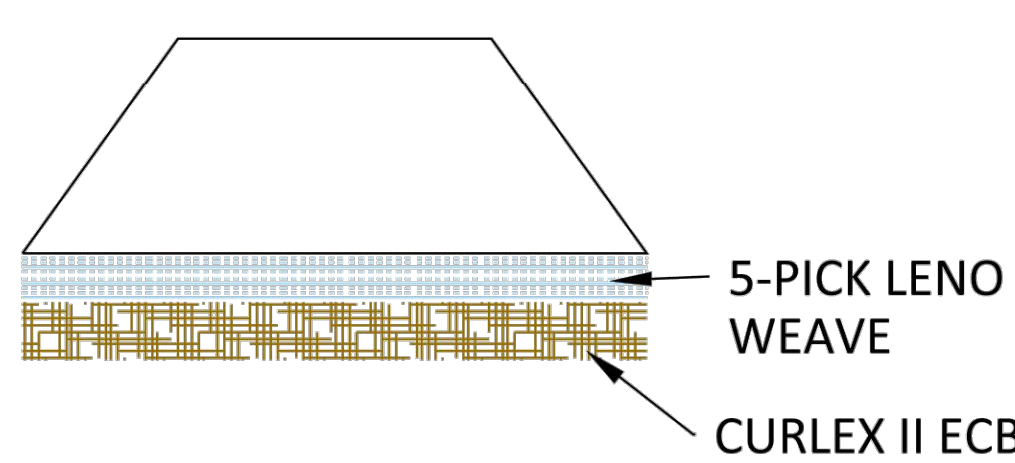
ISOMETRIC VIEW OF SLOPE AND ANCHOR TRENCHES



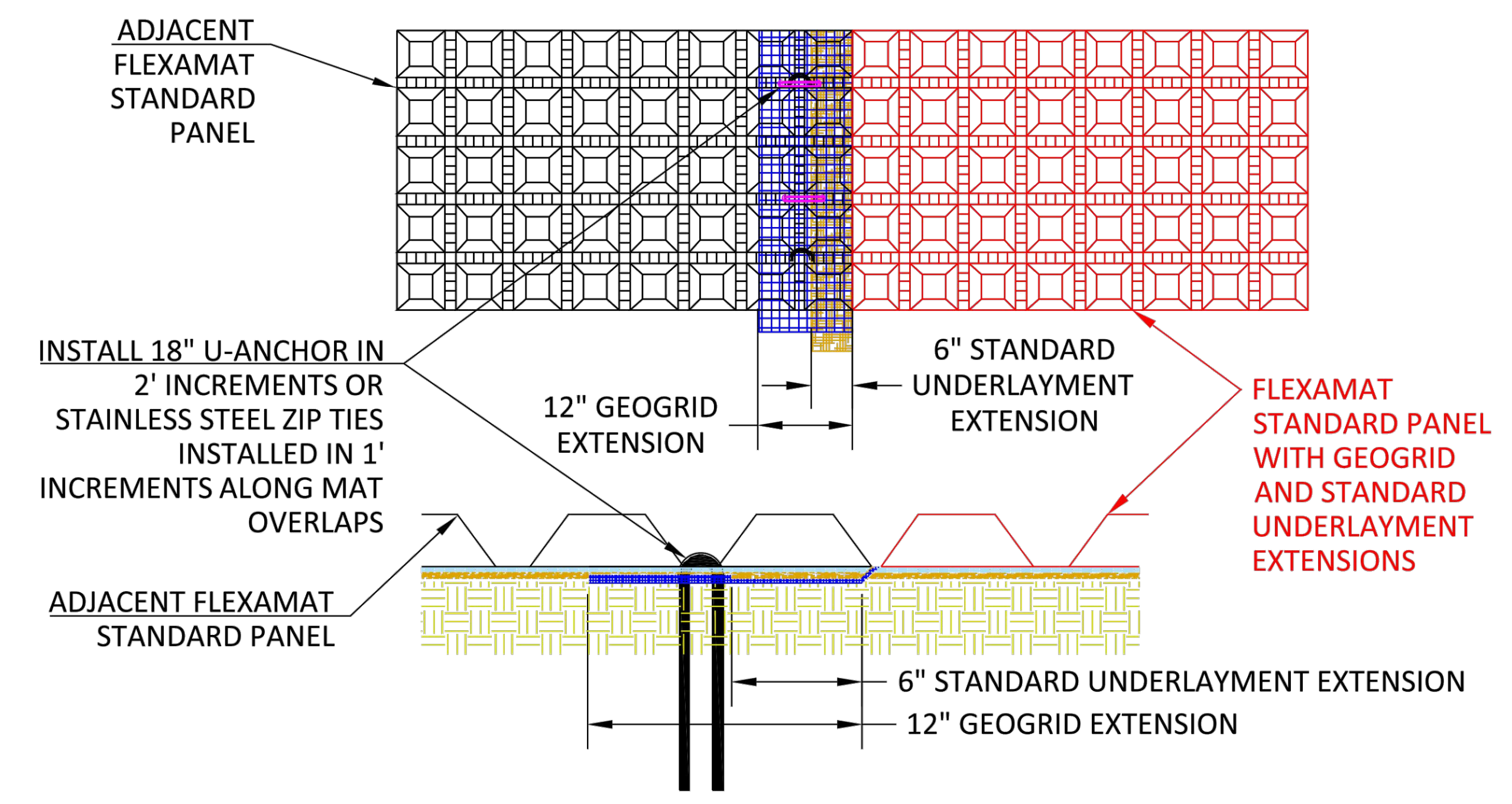
ANCHOR TRENCH



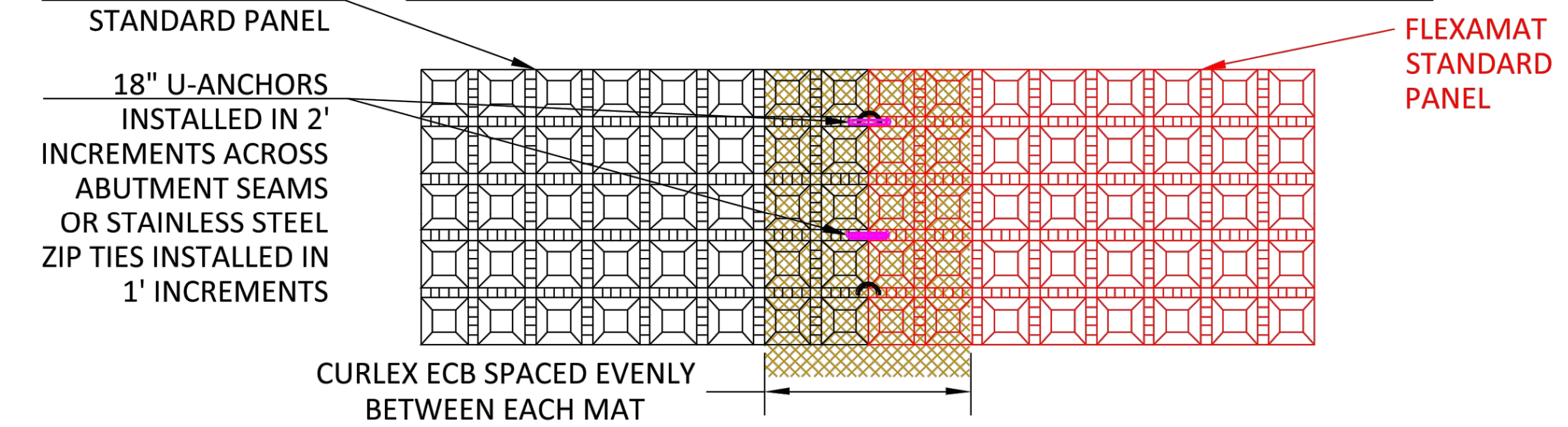
FLEXAMAT STANDARD UNDERLAYMENT



ABUTMENT METHOD FOR SEAMS WITH EXTENSIONS



ABUTMENT METHOD FOR SEAMS WITHOUT EXTENSIONS



FLEXAMAT STANDARD - SLOPE ARMORING

CONSTRUCTION NOTES:

1. AN ENGINEER OR MANUFACTURERS REPRESENTATIVE SHALL BE ONSITE FOR THE START OF THE INSTALLATION.
2. ALL SUBGRADE SURFACES PREPARED FOR PLACEMENT OF MATS SHALL BE SMOOTH AND FREE OF ALL ROCKS, STICKS, ROOTS, OTHER PROTRUSIONS, OR DEBRIS OF ANY KIND.
3. PRIOR TO FLEXAMAT STANDARD INSTALLATION SEED AND FERTILIZE SUBGRADE WITH SITE SPECIFIC SEED MIX IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS.
4. INSTALL FLEXAMAT STANDARD ROLLS THAT ARE CONTINUOUS FOR ENTIRE SLOPE LENGTH. FOR SLOPES LONGER THAN 16', USE MATS WITH EXTENSIONS CUT TO THE LENGTH OF THE SLOPE. INSTALL MATS TO THAT THE MATTING EXTENDS PAST THE CREST OF SLOPE AND INTO AN 18" ANCHOR TRENCH.
 - 4.1. FOR ARMORED SLOPE LENGTHS 16' OR LESS, INSTALL CURLEX ECB EQUALLY UNDER ADJACENT MATS. SECURE SEAM WITH #3 REBAR 18" U-ANCHORS IN 2' INCREMENTS THE LENGTH OF THE ABUTMENT.
 - 4.2. ARMORED SLOPE LENGTHS LONGER THAN 16', INSTALL NEXT MAT OVER EXTENSIONS.
5. INSTALL SUBSEQUENT MATS OVER THE GEOGRID EXTENSION AND STANDARD UNDERLAYMENT EXTENSION OF THE PREVIOUSLY INSTALLED MAT. ENSURE THE GEOGRID AND STANDARD UNDERLAYMENT EXTENSIONS ARE LAYING FLAT ON THE SUBGRADE BEFORE INSTALLING ADJACENT MAT OVER THE EXTENSIONS.
6. INSTALL #3 REBAR 18" U-ANCHORS IN 2' INCREMENTS ACROSS THE GEOGRID AND STANDARD EXTENSION ABUTMENT. INSTALL ANCHORS PERPENDICULAR TO THE SLOPE DIRECTLY BEHIND FIRST ROW OF BLOCKS ON THE ADJACENT MAT.
7. AT THE END OF THE ARMORED SLOPE, IF HEAD CUT IS ANTICIPATED, EMBED THE MAT 18" IN A TERMINATION TRENCH. FILL AND COMPACT TERMINATION TRENCH WITH SUITABLE FILL.

MOTZ ENTERPRISES, INC.

Flexamat
 (513)772-6689
 Info@Flexamat.com
 Flexamat.com



REV - 1

REV	DATE	BY	DESCRIPTION
10	08-12-2024	MWF	CC010

SLOPE CONSTRUCTION DETAILS

LOVE'S TRAVEL STOP - S.R. 149 ROADWAY SLOPES

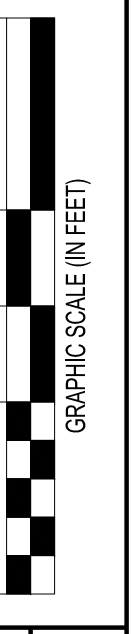
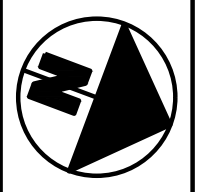
66320 BELMONT-MORRISTOWN ROAD
 ST. CLAIRSVILLE, OHIO

4885 SOUTH ASH AVENUE, SUITE 114
 TEMPE, AZ 85282
 PH: (602) 897-8200
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STATE OF OHIO
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DESIGNED BY:	JP
DRAWN BY:	MWF
APPVD. BY:	TMK
SCALE:	AS SHOWN
DATE:	9/06/24
JOB NO.:	65235270
ACAD NO.:	6 GR-6
SHEET NO.:	6 OF 6

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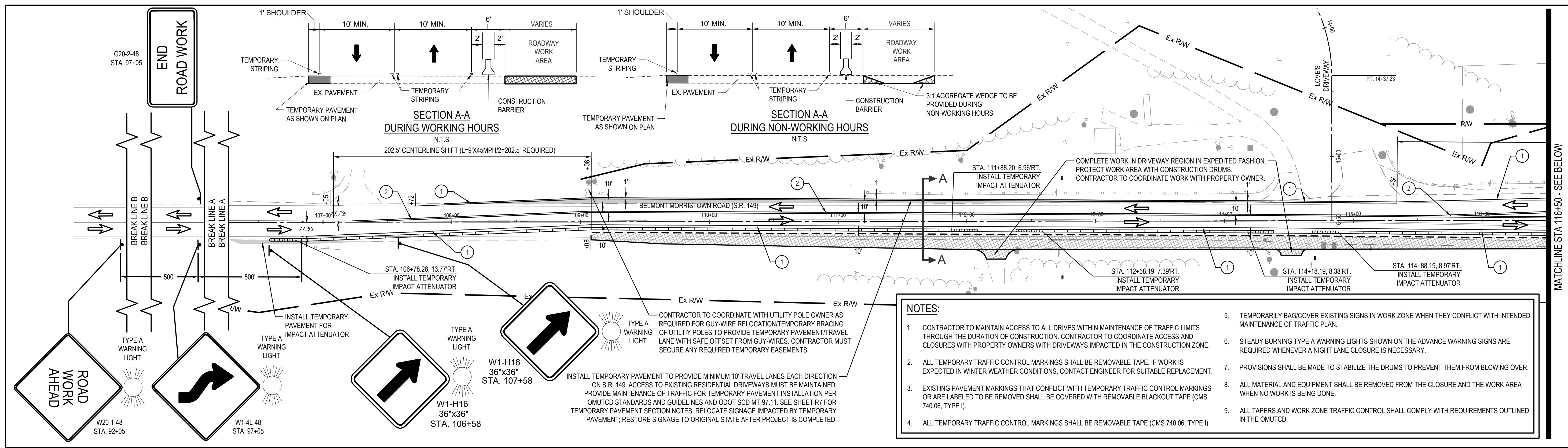


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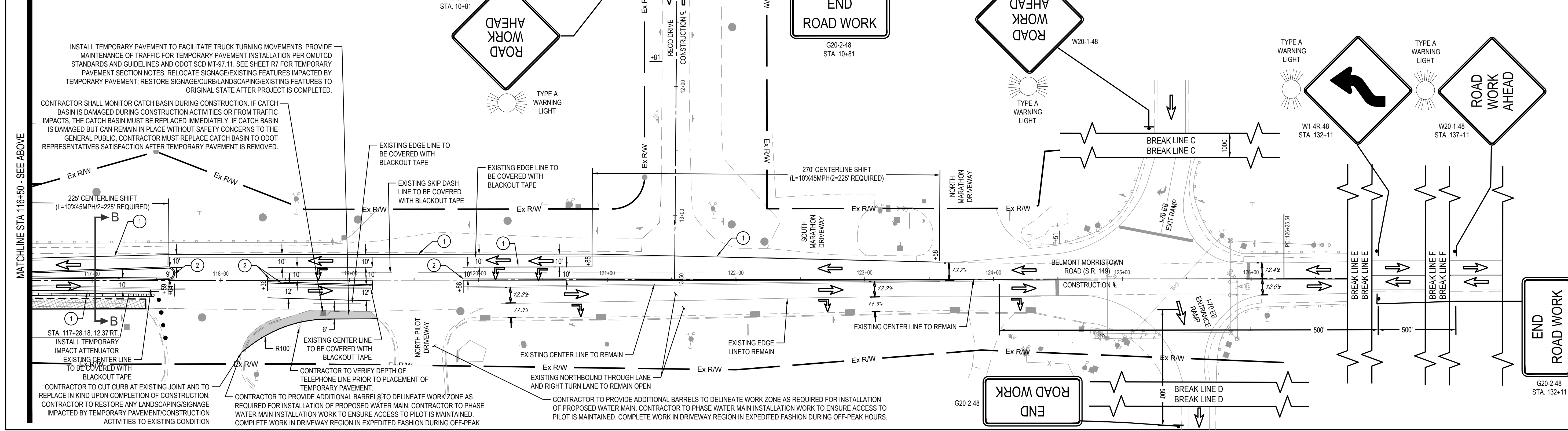
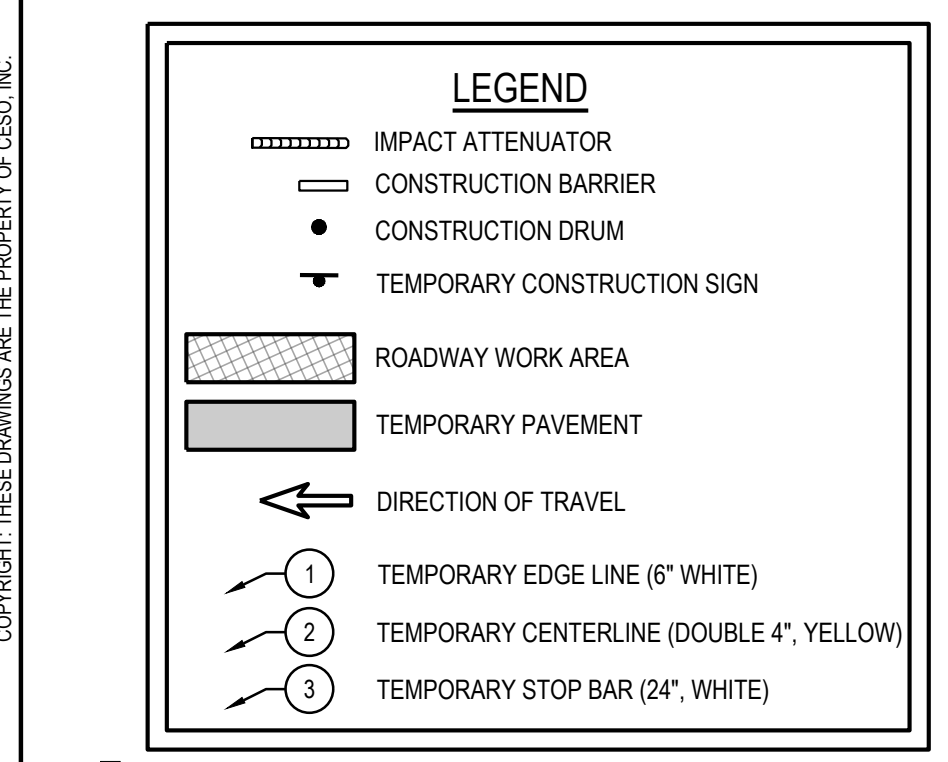
MAINTENANCE OF TRAFFIC - PHASE 1
S.R. 149

BEL-149-23.44

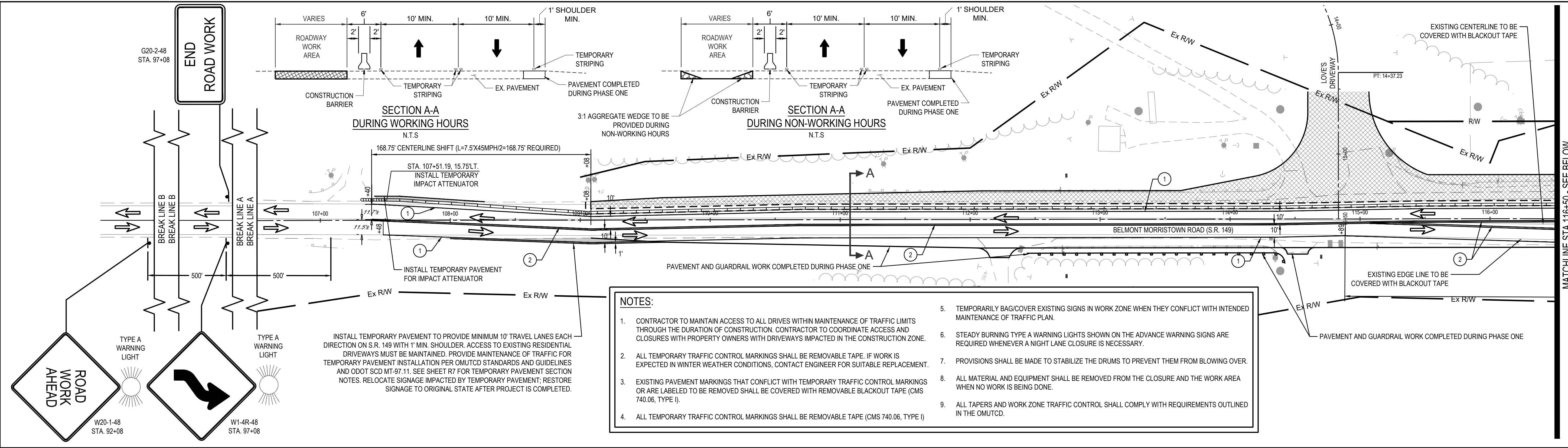
R8
R32



- NOTES:**
- CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVES WITHIN MAINTENANCE OF TRAFFIC LIMITS THROUGH THE DURATION OF CONSTRUCTION. CONTRACTOR TO COORDINATE ACCESS AND CLOSURES WITH PROPERTY OWNERS WITH DRIVEWAYS IMPACTED IN THE CONSTRUCTION ZONE.
 - ALL TEMPORARY TRAFFIC CONTROL MARKINGS SHALL BE REMOVABLE TAPE. IF WORK IS EXPECTED IN WINTER WEATHER CONDITIONS, CONTACT ENGINEER FOR SUITABLE REPLACEMENT.
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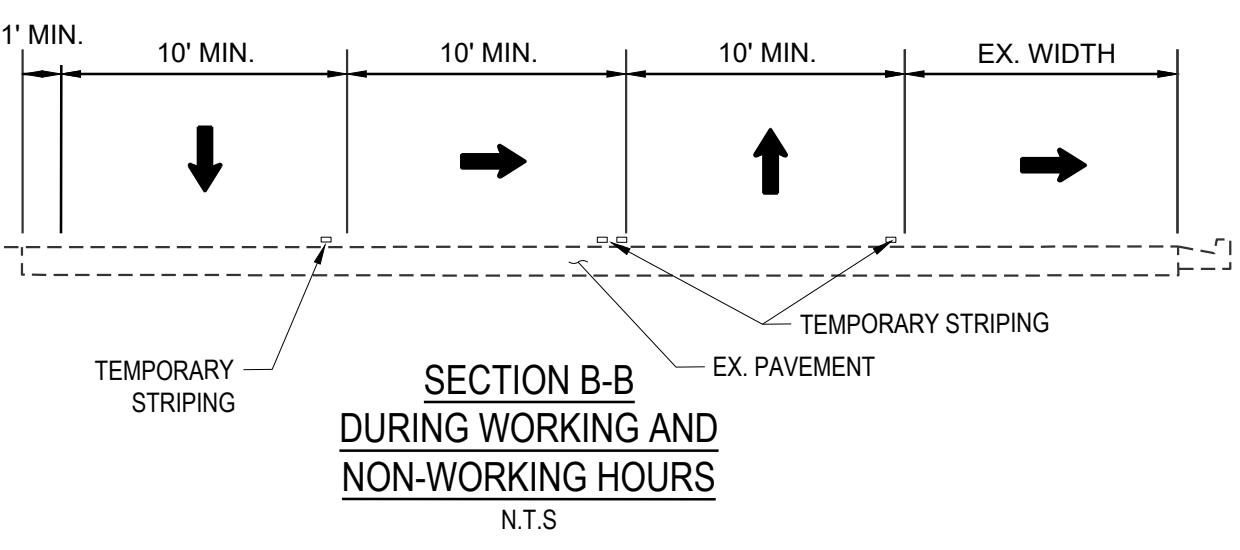
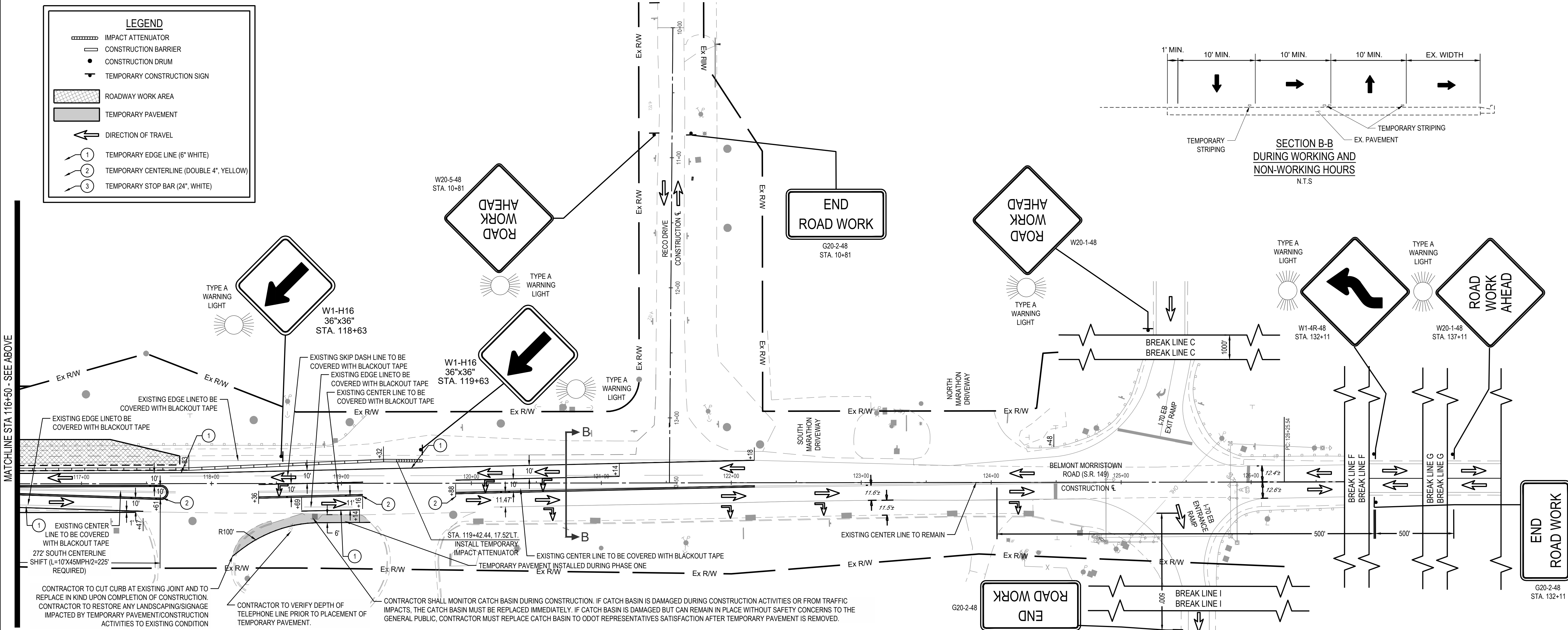
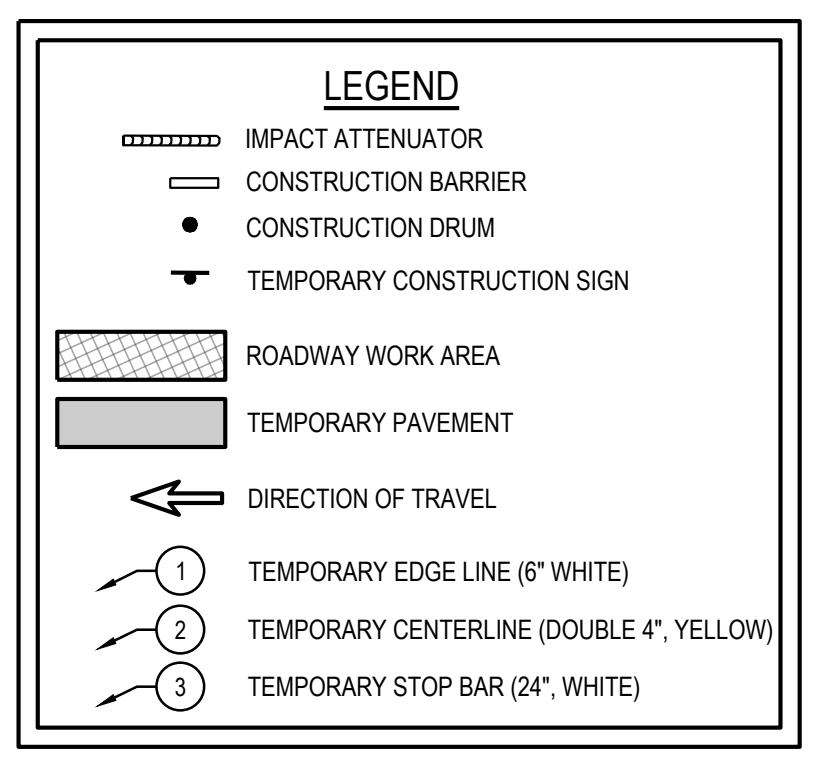
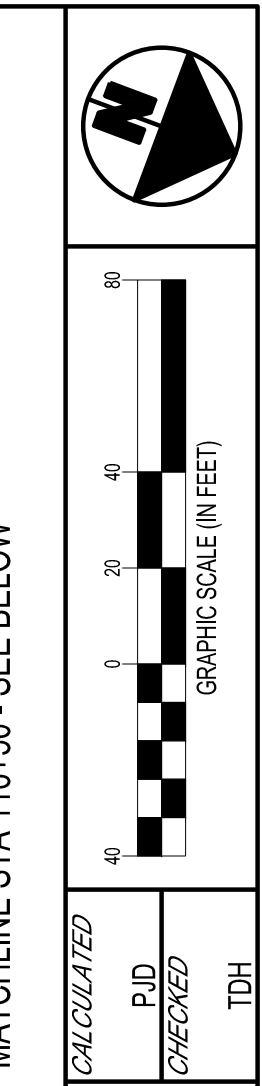


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INSTALL TEMPORARY PAVEMENT TO PROVIDE MINIMUM 10' TRAVEL LANES EACH DIRECTION ON S.R. 149 WITH 1' MIN. SHOULDER. ACCESS TO EXISTING RESIDENTIAL DRIVEWAYS MUST BE MAINTAINED. PROVIDE MAINTENANCE OF TRAFFIC FOR TEMPORARY PAVEMENT INSTALLATION PER OMTCD STANDARDS AND GUIDELINES AND ODOT SCD MT-97.11. SEE SHEET R7 FOR TEMPORARY PAVEMENT SECTION NOTES. RELOCATE SIGNAGE IMPACTED BY TEMPORARY PAVEMENT; RESTORE SIGNAGE TO ORIGINAL STATE AFTER PROJECT IS COMPLETED.



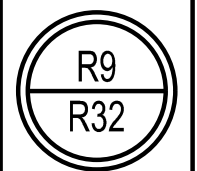
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MATCHLINE STA 116+50 - SEE ABOVE

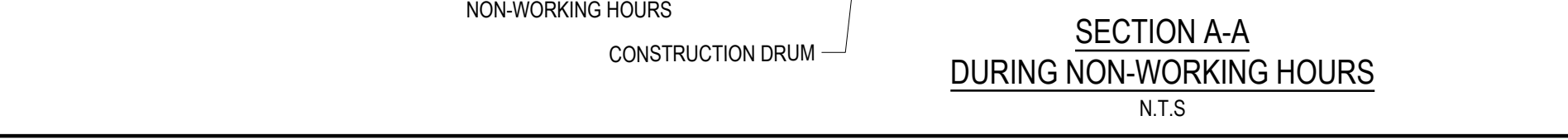
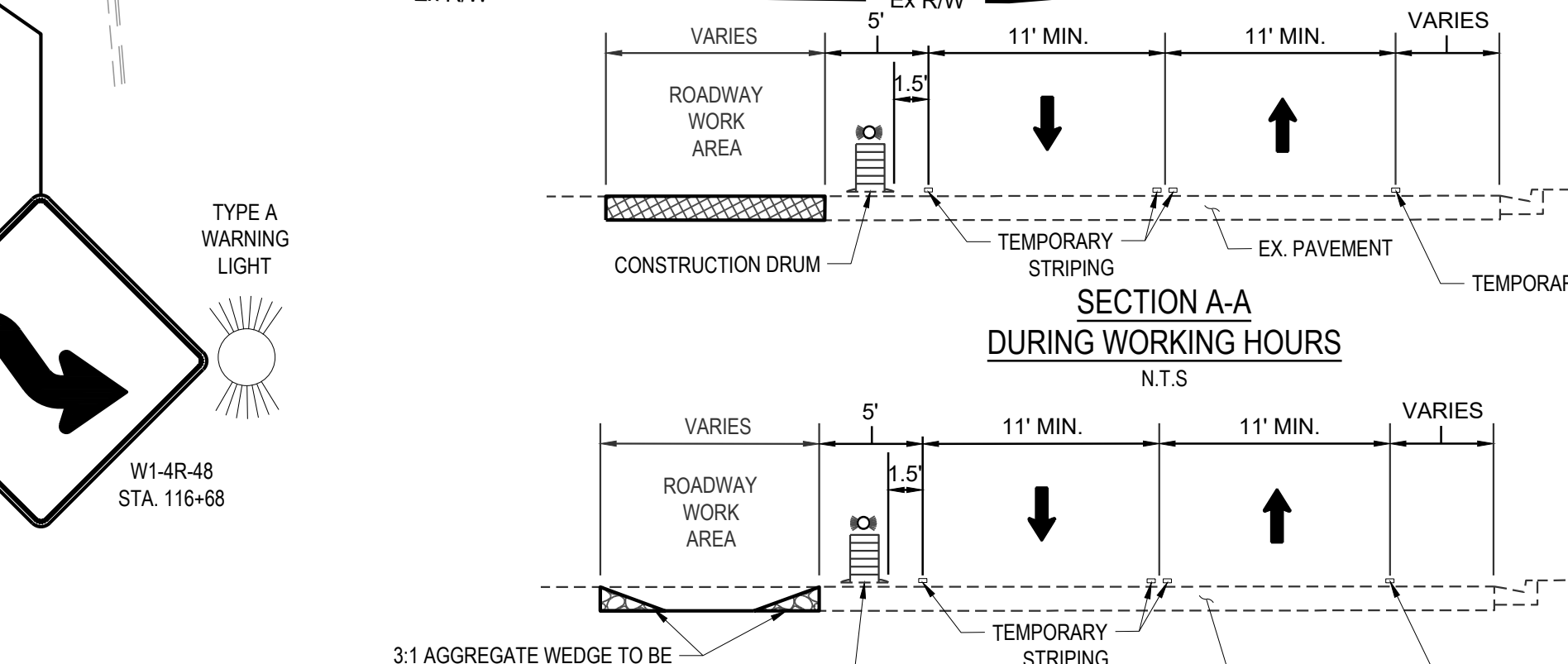
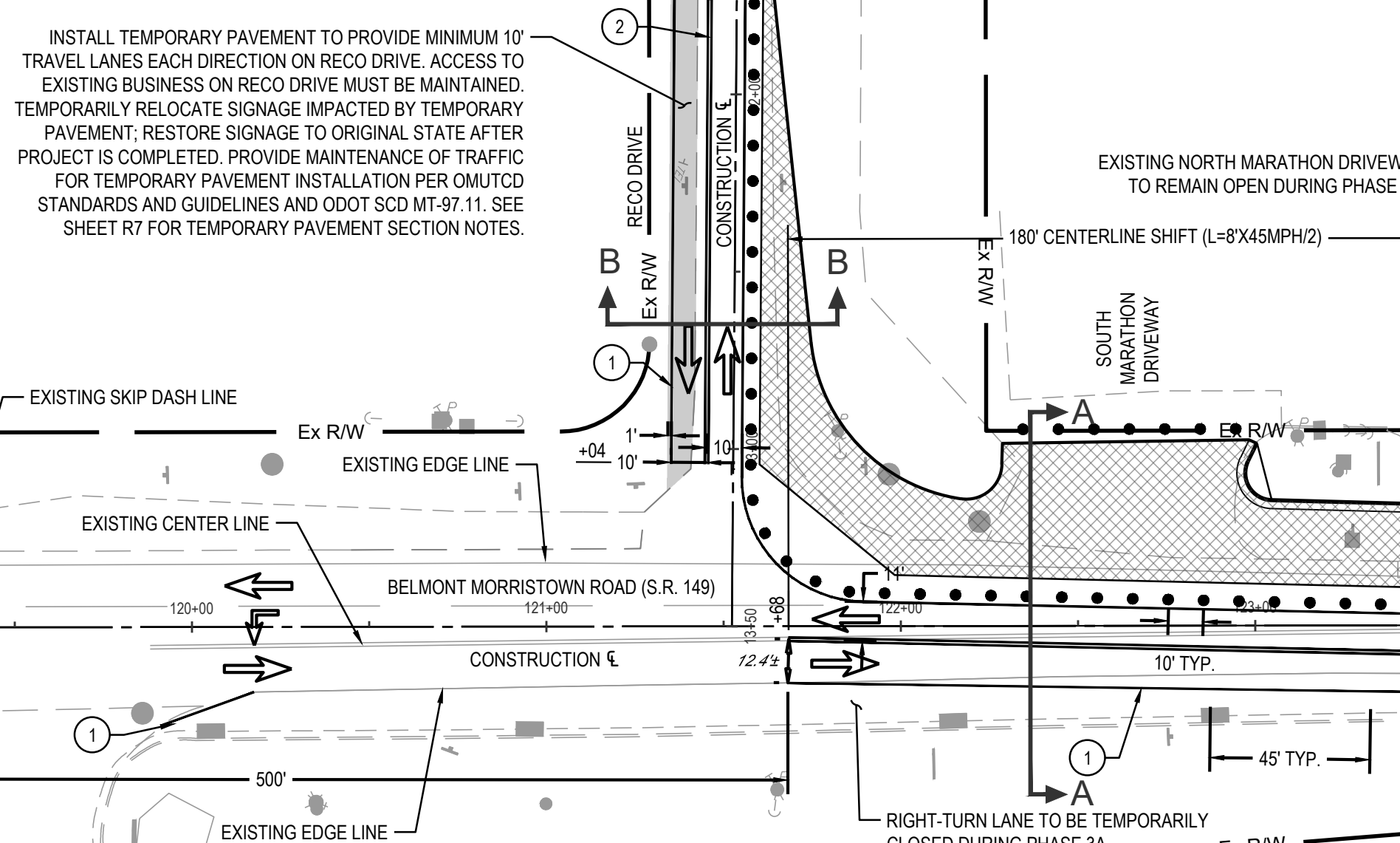
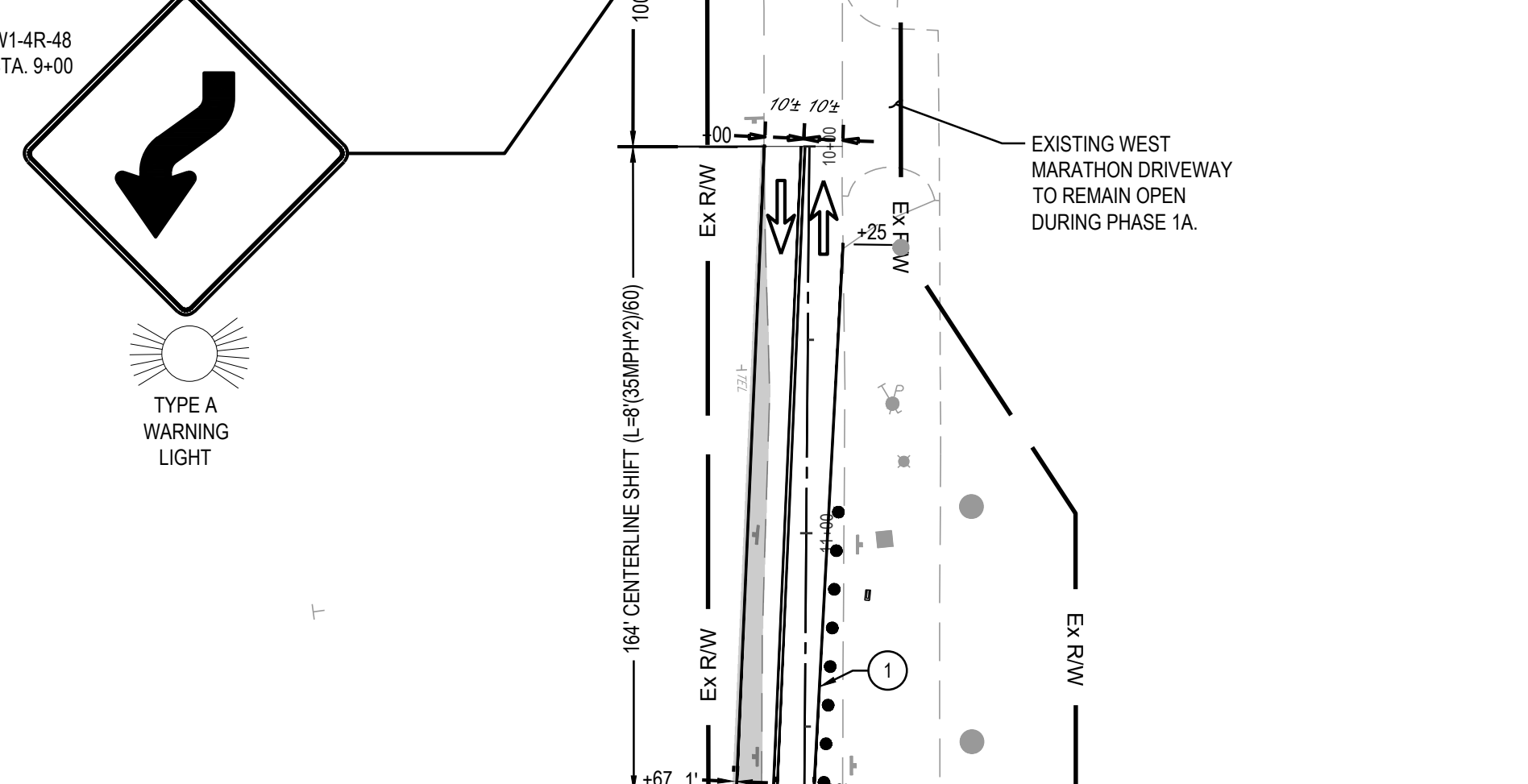
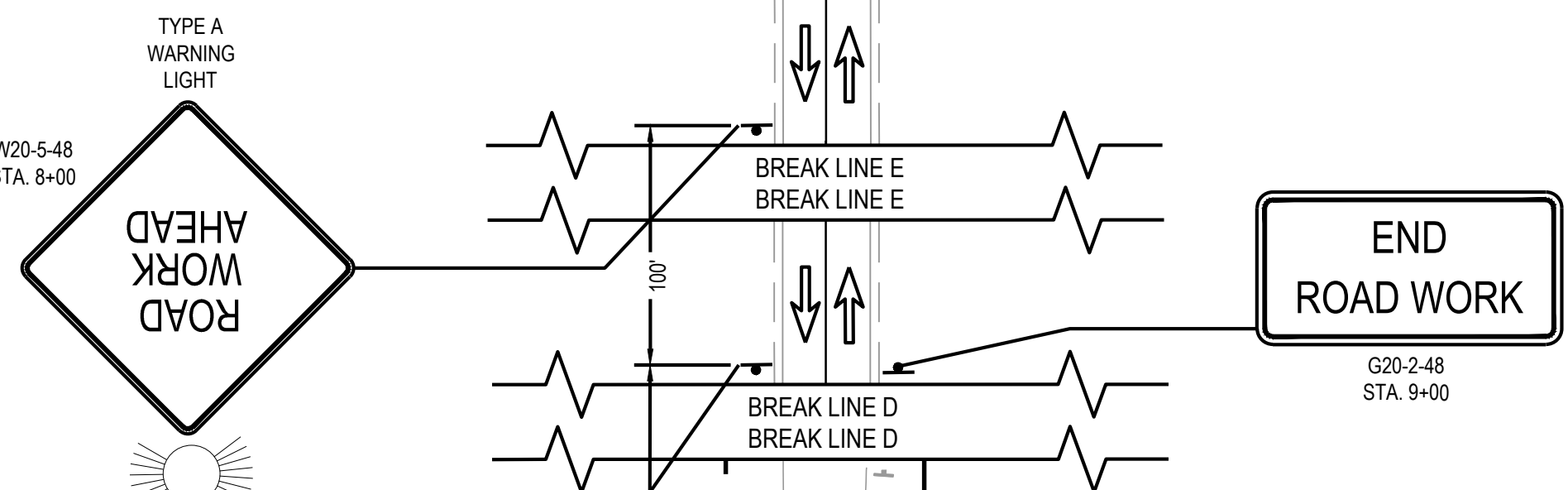
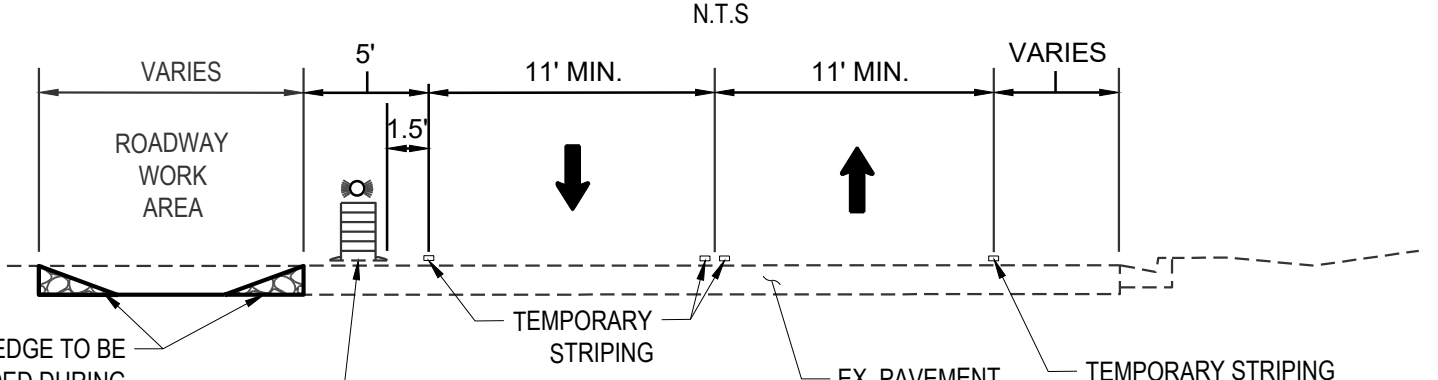
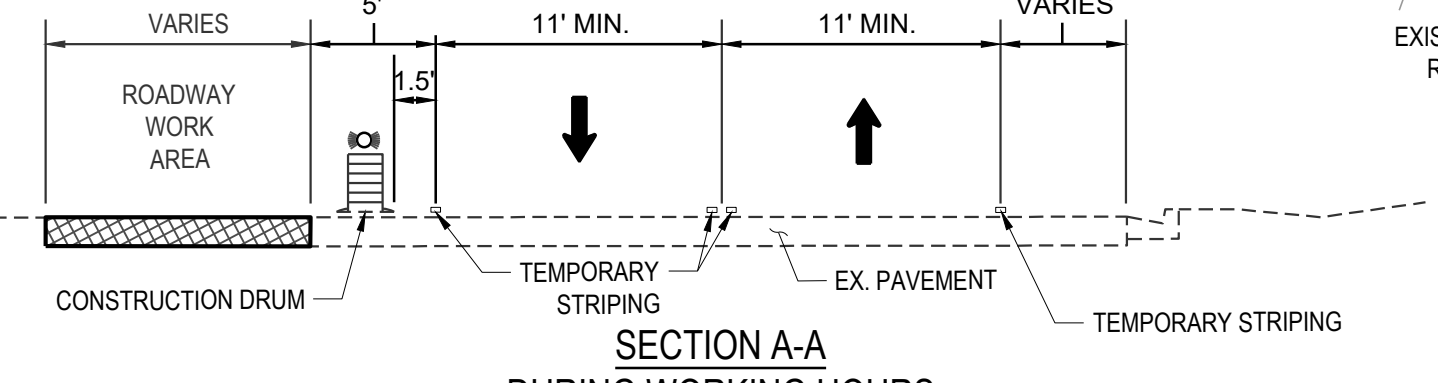
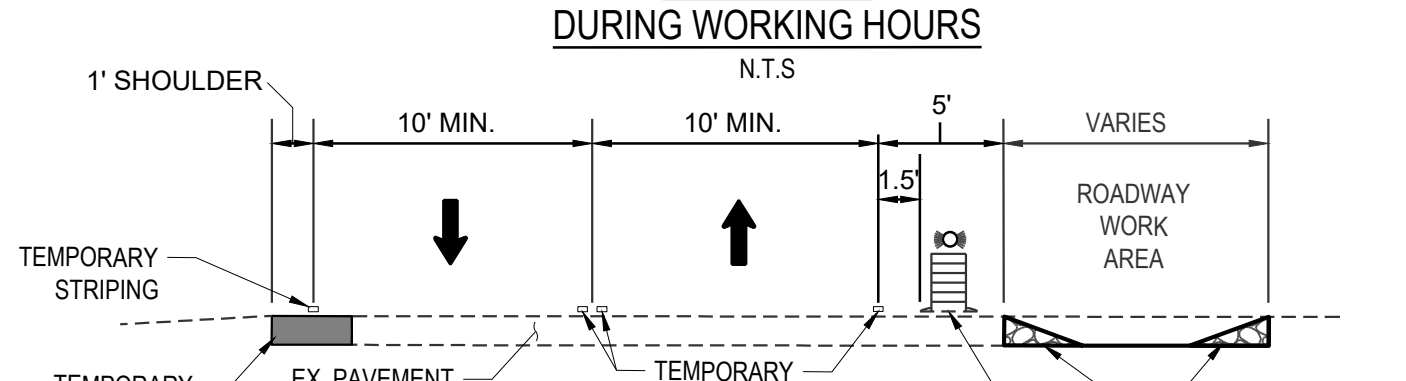
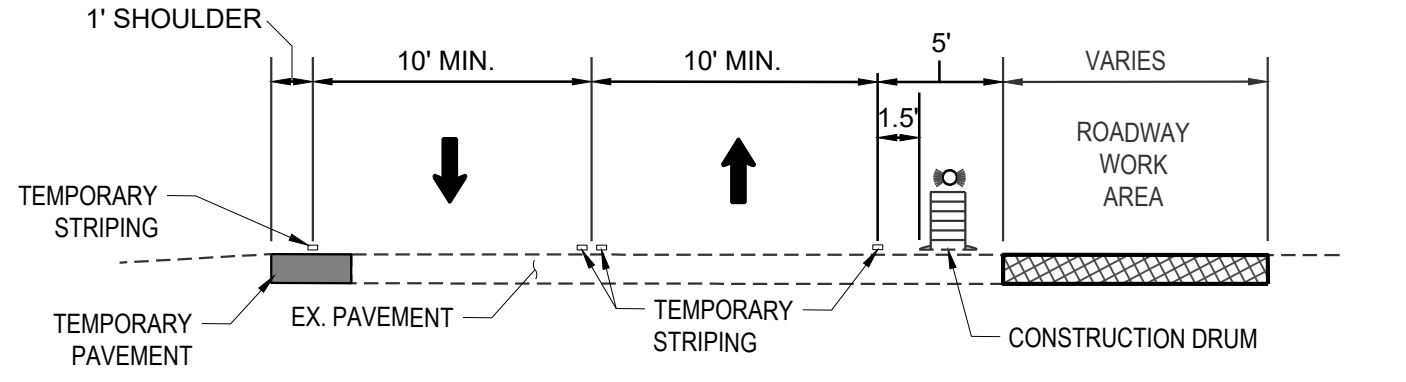
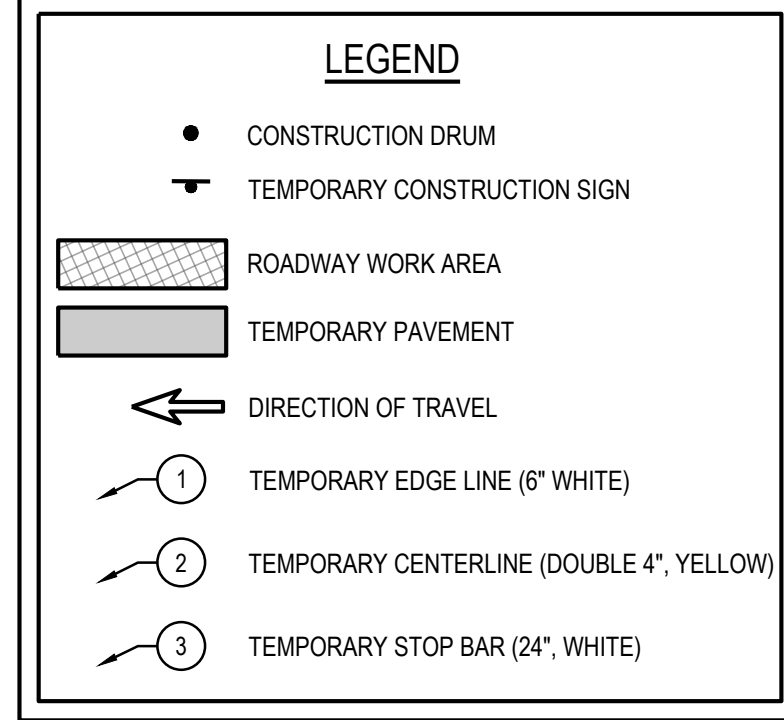
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MAINTENANCE OF TRAFFIC - PHASE 2
S.R. 149

BEL-149-23.44

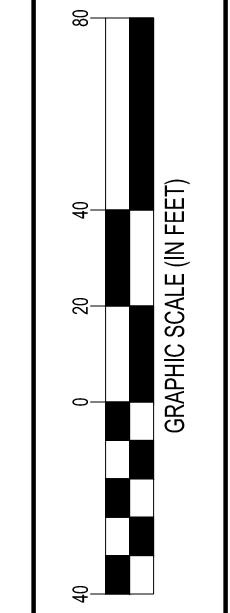
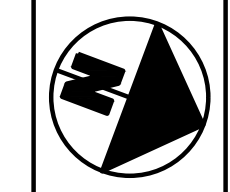


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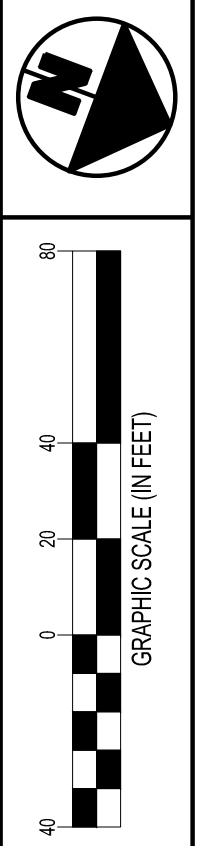
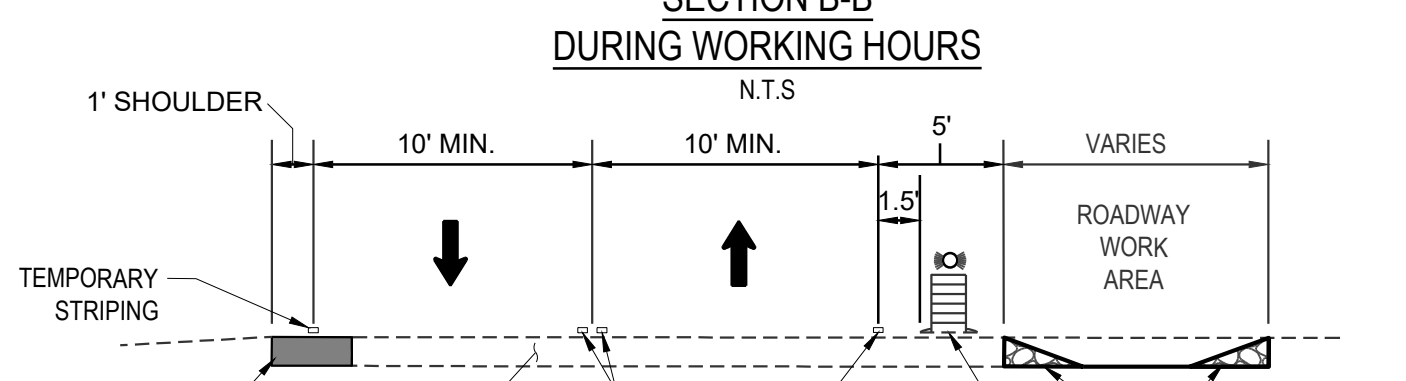
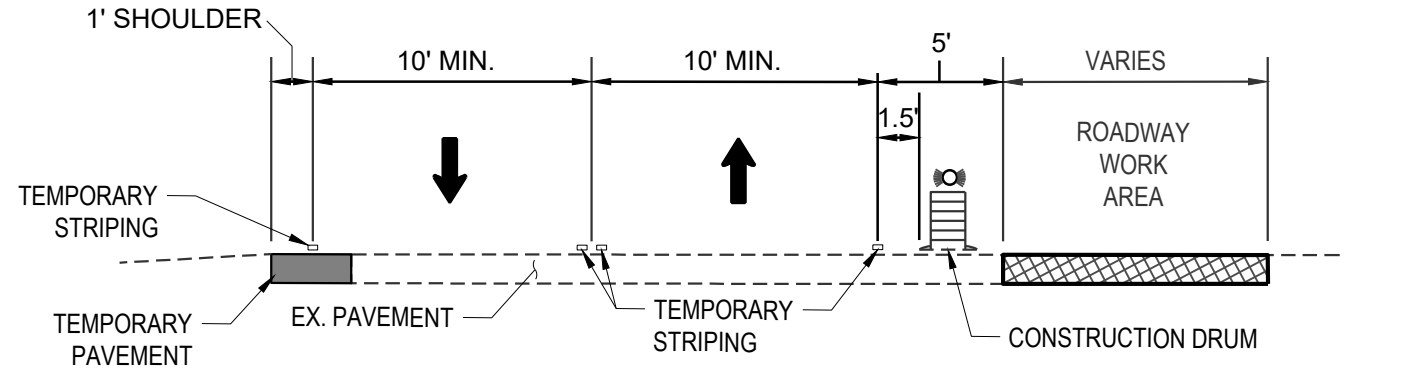
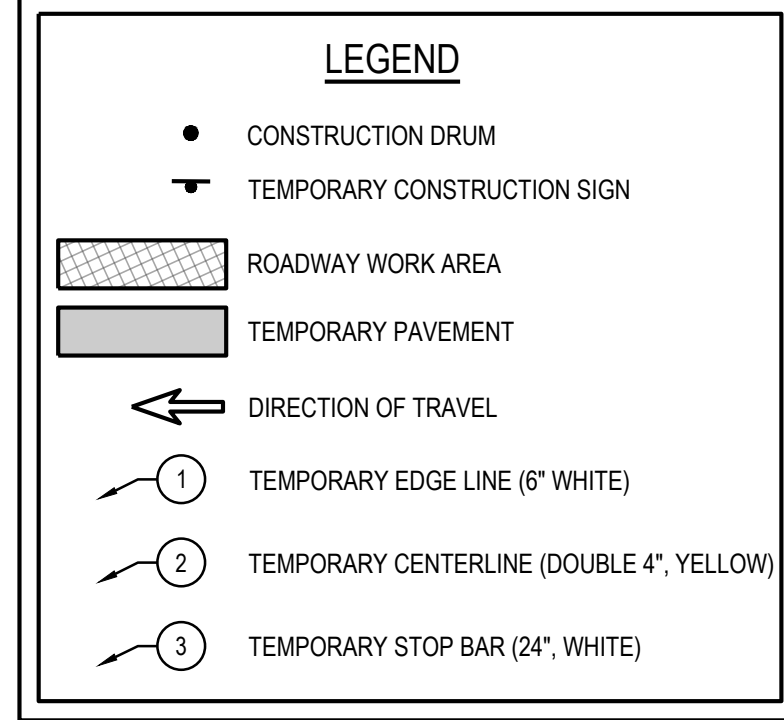
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MAINTENANCE OF TRAFFIC - PHASE 3A
S.R. 149

BEL-149-23.44

R10
R32

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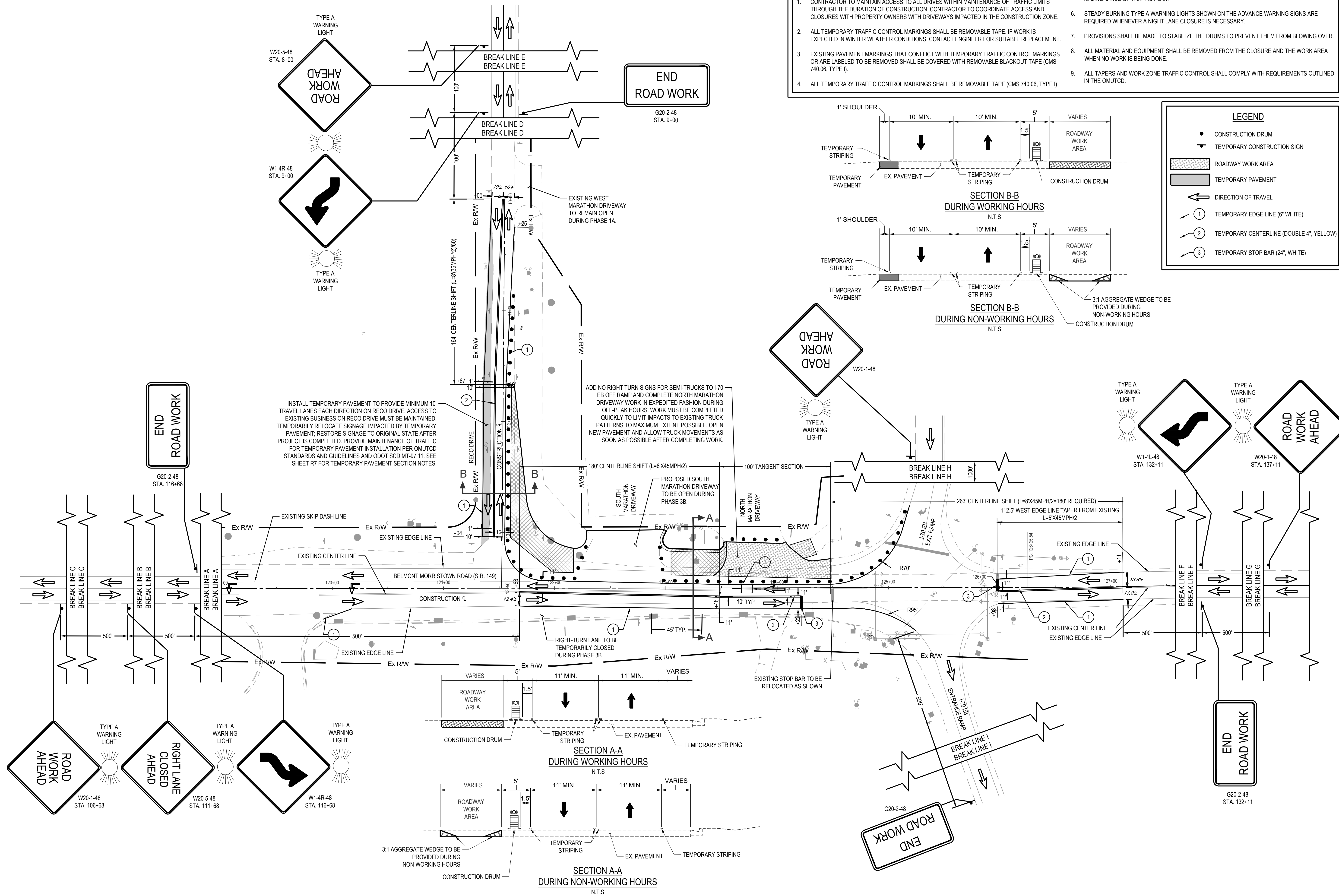
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MAINTENANCE OF TRAFFIC - PHASE 3B
S.R. 149

BEL-149-23.44

R11
R32

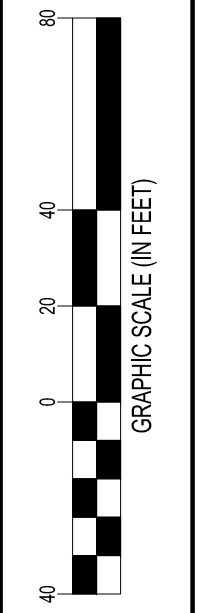
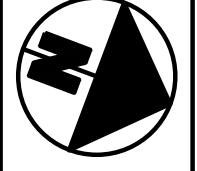
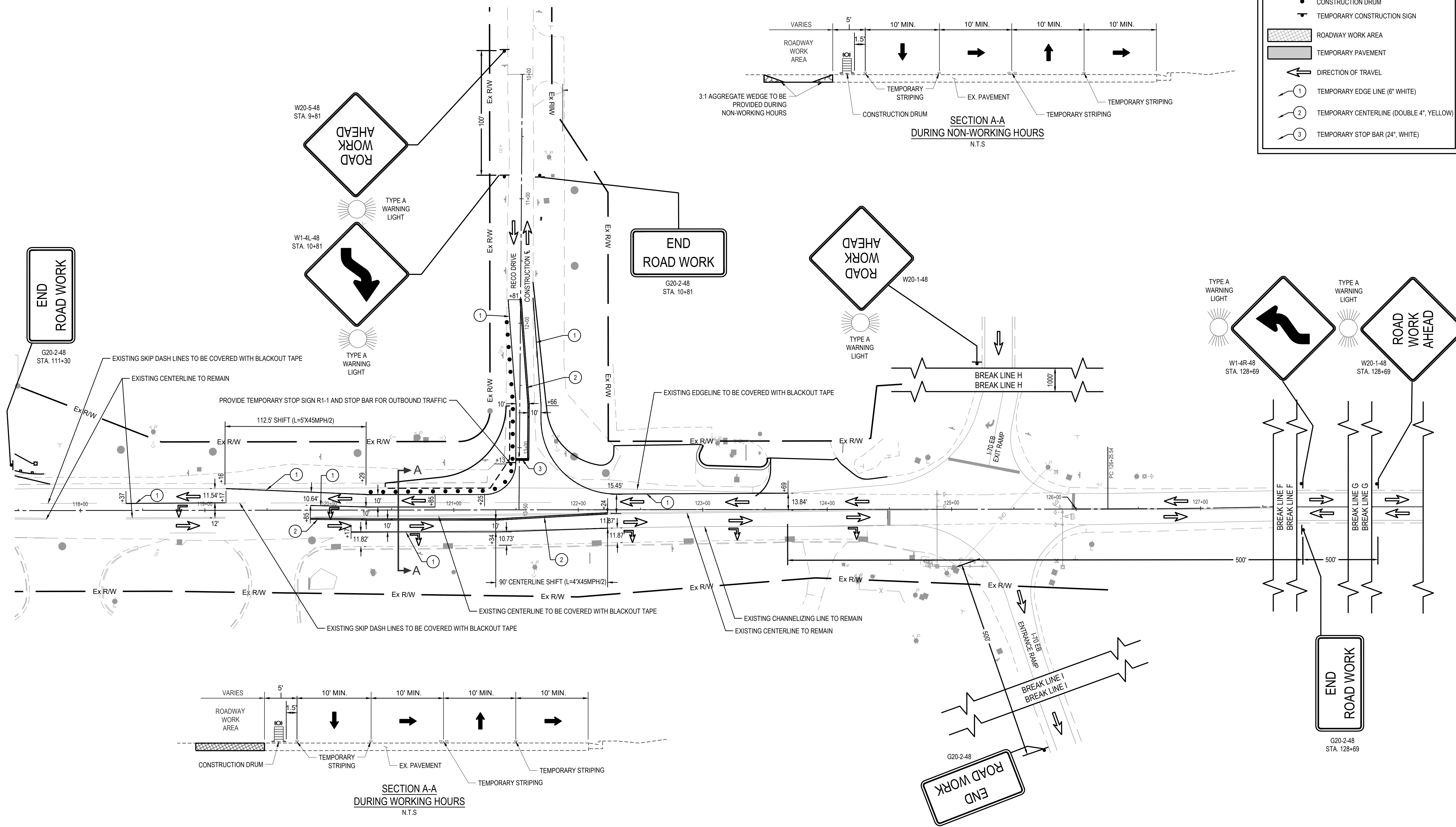
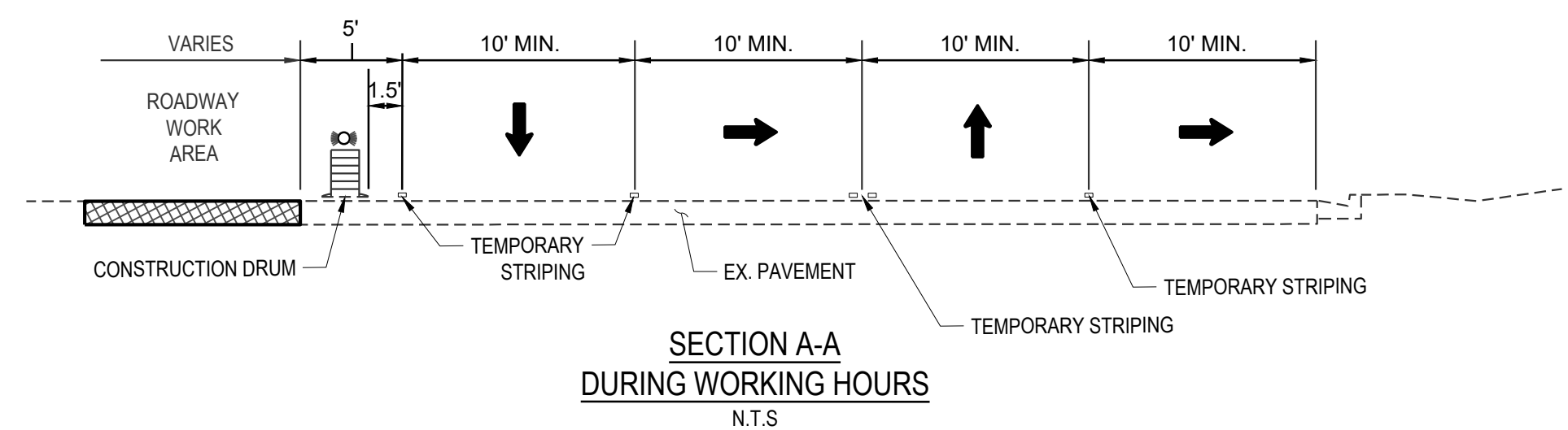
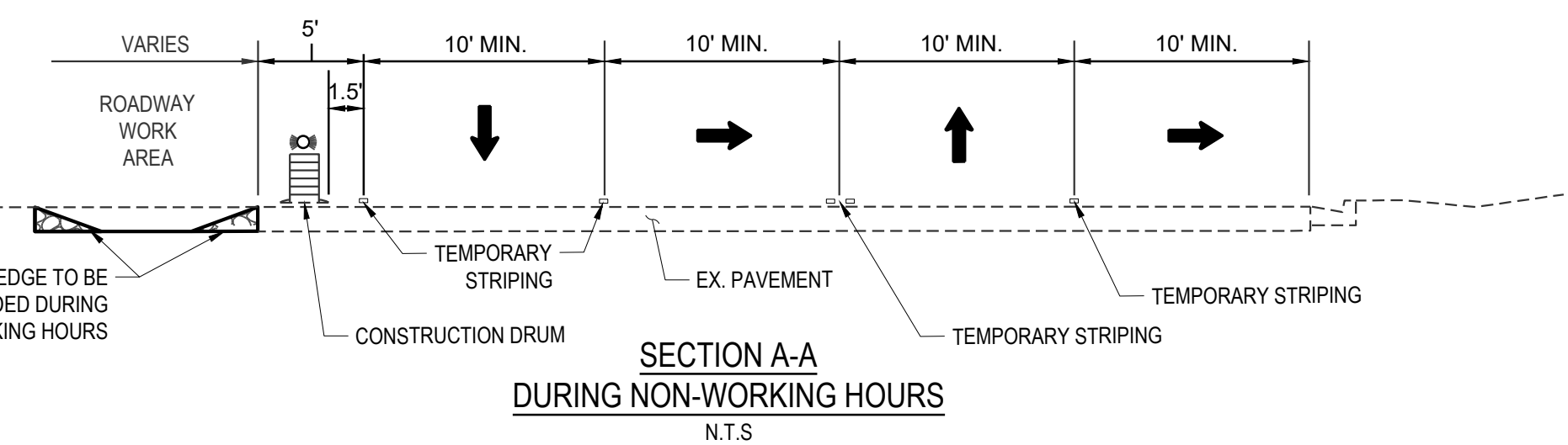
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LEGEND

- CONSTRUCTION DRUM
- ⬇️ TEMPORARY CONSTRUCTION SIGN
- ▨ ROADWAY WORK AREA
- ▩ TEMPORARY PAVEMENT
- ➔ DIRECTION OF TRAVEL
- ① TEMPORARY EDGE LINE (6" WHITE)
- ② TEMPORARY CENTERLINE (DOUBLE 4", YELLOW)
- ③ TEMPORARY STOP BAR (24", WHITE)



CALCULATED
P.I.D.
CHECKED
TDH

MAINTENANCE OF TRAFFIC - PHASE 3C
S.R. 149

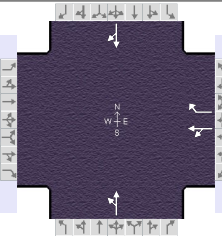
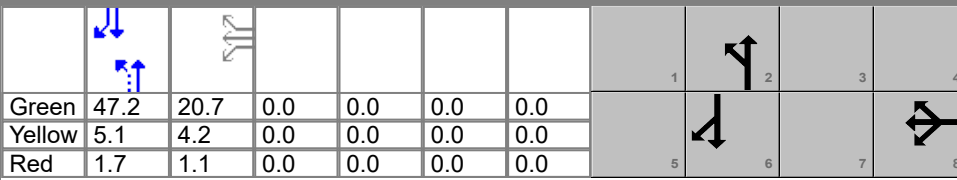
BEL-149-23.44

R12
R32

APPENDIX F
2043 DESIGN YEAR TRAFFIC SCENARIO
CAPACITY ANALYSIS SUMMARY SHEETS

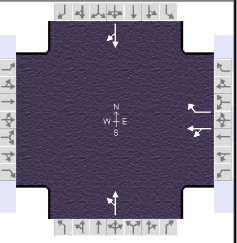
CONDITION 1

HCS Signalized Intersection Input Data

General Information					Intersection Information										
Agency	CESO				Duration, h	0.250									
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other								
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.81								
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00								
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD AM Peak - Int 1 & Int 2.xus											
Project Description	2043 Build Condition 1 Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h							280	1	210	160	330			288	55
Signal Information							Cycle, s		80.0	Reference Phase	2				
Offset, s		0					Reference Point	End							
Uncoordinated	No		Simult. Gap E/W				On								
Force Mode	Fixed		Simult. Gap N/S				On								
Green				47.2	20.7	0.0	0.0	0.0	0.0						
Yellow				5.1	4.2	0.0	0.0	0.0	0.0						
Red				1.7	1.1	0.0	0.0	0.0	0.0						
Traffic Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h							280	1	210	160	330			288	55
Initial Queue (Q _b), veh/h							0	0	0	0	0			0	0
Base Saturation Flow Rate (s ₀), veh/h							1750	1750	1750	1750	1750			1750	1750
Parking (N _m), man/h							None			None			None		
Heavy Vehicles (P _{HV}), %							17	17		13			11		
Ped / Bike / RTOR, /h				0	0		0	0	0	0	0		0	0	0
Buses (N _b), buses/h							0	0	0	0	0	0	0	0	0
Arrival Type (AT)							3	3	3	3	3		3	3	
Upstream Filtering (I)							1.00	1.00	1.00	0.85	0.85		1.00	1.00	
Lane Width (W), ft							12.0	12.0		12.0		12.0			
Turn Bay Length, ft							1145	350		400		635			
Grade (P _g), %					0		0			0		0			
Speed Limit, mi/h							35	35	35	45	45		45	45	
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Maximum Green (G _{max}) or Phase Split, s							26.0		54.0		54.0				
Yellow Change Interval (Y), s							4.2		5.1		5.1				
Red Clearance Interval (R _c), s							1.1		1.7		1.7				
Minimum Green (G _{min}), s							10		20		20				
Start-Up Lost Time (l _t), s						2.0	2.0	2.0	2.0		2.0				
Extension of Effective Green (e), s						2.0	2.0	2.0	2.0		2.0				
Passage (P _T), s							2.0		2.0		2.0				
Recall Mode							Off		Min		Min				
Dual Entry							Yes		Yes		Yes				
Walk (Walk), s					0.0		0.0		0.0		0.0				
Pedestrian Clearance Time (P _C), s					0.0		0.0		0.0		0.0				
Multimodal Information				EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0	0.0	No	25.0			
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0			
Street Width / Island / Curb, ft					0		0.0	0	No	0.0	0	No	0.0		No
Width Outside / Bike Lane / Shoulder, ft							12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking				No			No	0.50		No	0.50			0.50	

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD AM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				280	1	210	160	330			288	55

Signal Information																		
Cycle, s	80.0	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	No	Simult. Gap E/W	On	Green	47.2	20.7	0.0	0.0	0.0	0.0	1		2		3		4	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5		6		7		8	
				Red	1.7	1.1	0.0	0.0	0.0	0.0								

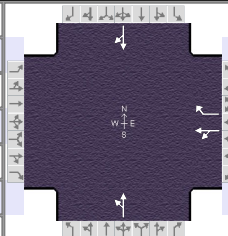
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				8		2		6
Case Number				11.0		8.0		8.0
Phase Duration, s				26.0		54.0		54.0
Change Period, (Y+R _c), s				5.3		6.8		6.8
Max Allow Headway (MAH), s				3.2		0.0		0.0
Queue Clearance Time (g _s), s				20.7				
Green Extension Time (g _e), s				0.0		0.0		0.0
Phase Call Probability				1.00				
Max Out Probability				1.00				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h					347	259		551				423
Adjusted Saturation Flow Rate (s), veh/h/ln					1446	1286		1060				1555
Queue Service Time (g _s), s					18.7	15.0		28.1				12.3
Cycle Queue Clearance Time (g _c), s					18.7	15.0		40.3				12.3
Green Ratio (g/C)					0.26	0.26		0.59				0.59
Capacity (c), veh/h					374	333		685				917
Volume-to-Capacity Ratio (X)					0.927	0.779		0.803				0.462
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)					14.0	9.1		16.1				6.6
Queue Storage Ratio (RQ) (95 th percentile)					0.35	0.74		1.11				0.28
Uniform Delay (d ₁), s/veh					28.9	27.5		24.0				9.2
Incremental Delay (d ₂), s/veh					28.4	10.2		8.3				1.7
Initial Queue Delay (d ₃), s/veh					0.0	0.0		0.0				0.0
Control Delay (d), s/veh					57.3	37.8		32.4				10.9
Level of Service (LOS)					E	D		C				B
Approach Delay, s/veh / LOS	0.0			49.0		D	32.4		C	10.9		B
Intersection Delay, s/veh / LOS				33.0						C		

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.72	B	1.72	B	1.65	B	1.36	A
Bicycle LOS Score / LOS			1.49	A	1.49	A	1.19	A

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD AM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				280	1	210	160	330			288	55

Signal Information														
Cycle, s	80.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	47.2	20.7	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0				
				Red	1.7	1.1	0.0	0.0	0.0	0.0				

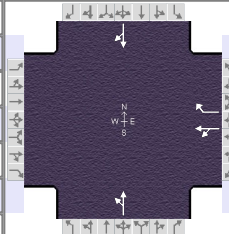
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})				0.953	0.953		0.674	0.674		1.000	0.972	
Right-Turn Adjustment Factor (f_{RT})						0.000	0.847		0.000	0.674		0.972
Left-Turn Pedestrian Adjustment Factor (f_{LPB})				1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})						1.000			1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h				1441	5	1286	346	714	0	0	1306	249
Proportion of Vehicles Arriving on Green (P)	0.00	0.00	0.00	0.26	0.26	0.26	0.30	0.46	0.00	0.00	0.59	0.59
Incremental Delay Factor (k)					0.43	0.30		0.50			0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)				4.0		6.8		6.8
Green Ratio (g/C)				0.26		0.59		0.59
Permitted Saturation Flow Rate (s_p), veh/h/ln				0		979		1028
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						0		1600
Permitted Effective Green Time (g_p), s				0.0		47.2		0.0
Permitted Service Time (g_u), s				0.0		34.9		0.0
Permitted Queue Service Time (g_{ps}), s						28.1		
Time to First Blockage (g_t), s				0.0		4.0		47.2
Queue Service Time Before Blockage (g_{ts}), s						4.0		
Protected Right Saturation Flow (s_R), veh/h/ln				0				
Protected Right Effective Green Time (g_R), s				0.0				

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000				
Pedestrian F_s / F_{delay}	0.000	0.148	0.000	0.148	0.000	0.076	0.000	0.076				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		45.16		46.55	1180.00	6.72	1180.00	6.72				
Bicycle F_w / F_v	-3.64		-3.64	1.00	-3.64	1.00	-3.64	0.70				

HCS Signalized Intersection Results Graphical Summary

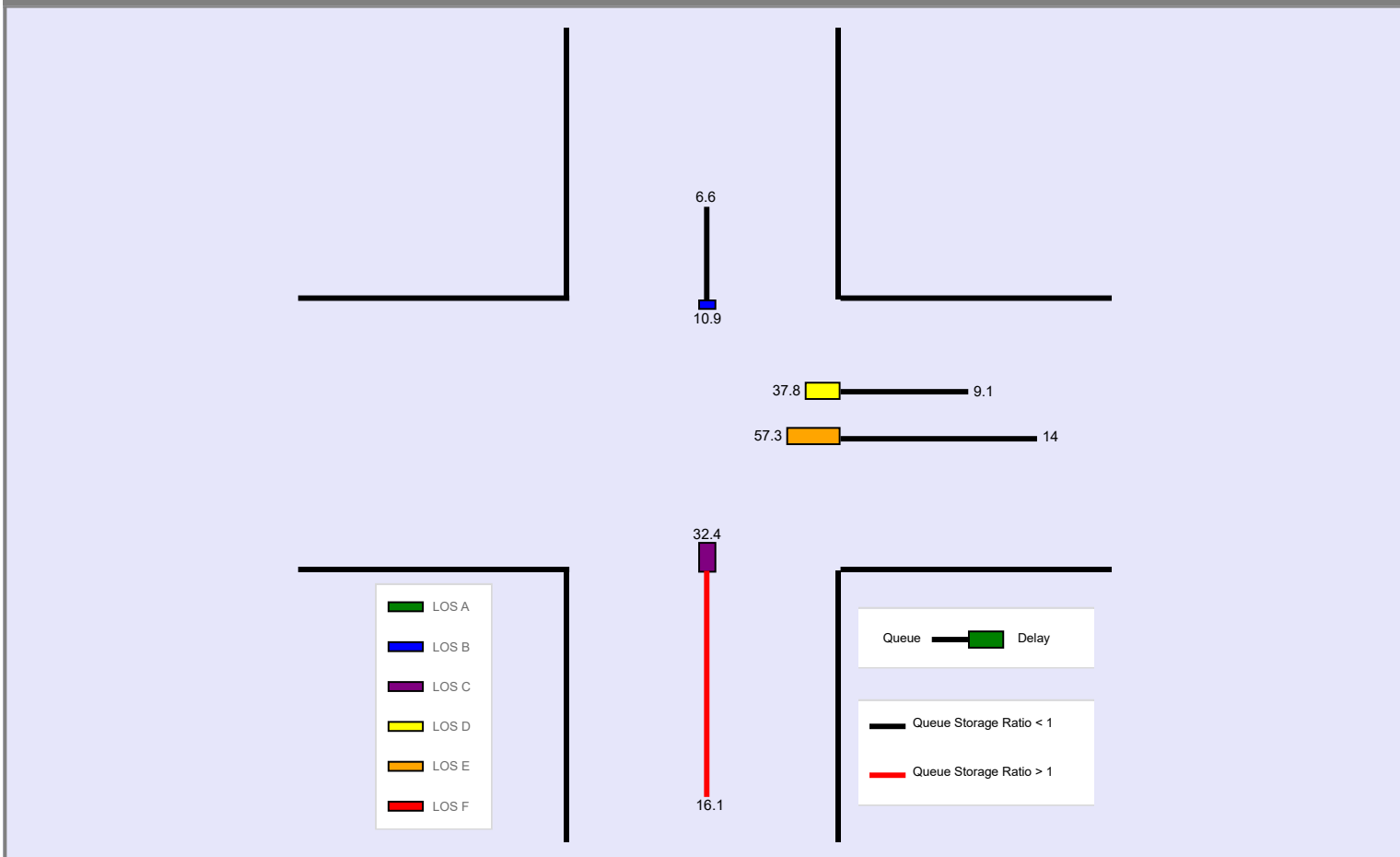
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD AM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				280	1	210	160	330			288	55

Signal Information																		
Cycle, s	80.0	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	No	Simult. Gap E/W	On	Green	47.2	20.7	0.0	0.0	0.0	0.0	1		2		3		4	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5		6		7		8	
				Red	1.7	1.1	0.0	0.0	0.0	0.0								

Movement Group Results	EB			WB			NB			SB					
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R			
Back of Queue (Q), ft/ln (95 th percentile)															
Back of Queue (Q), veh/ln (95 th percentile)						14.0	9.1		16.1			6.6			
Queue Storage Ratio (RQ) (95 th percentile)						0.35	0.74		1.11			0.28			
Control Delay (d), s/veh						57.3	37.8		32.4			10.9			
Level of Service (LOS)						E	D		C			B			
Approach Delay, s/veh / LOS	0.0			49.0			D			32.4			C		
Intersection Delay, s/veh / LOS							33.0			C					

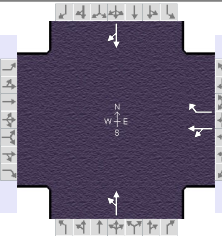
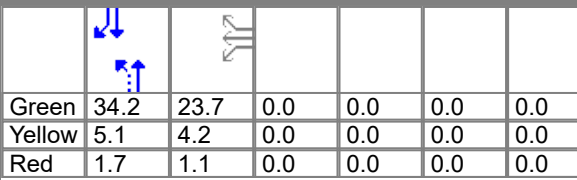
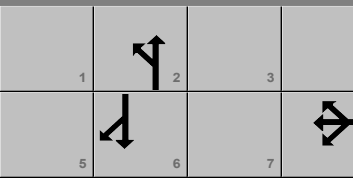


--- Messages ---

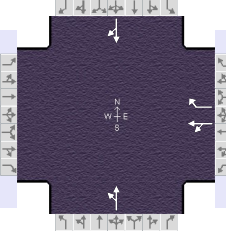
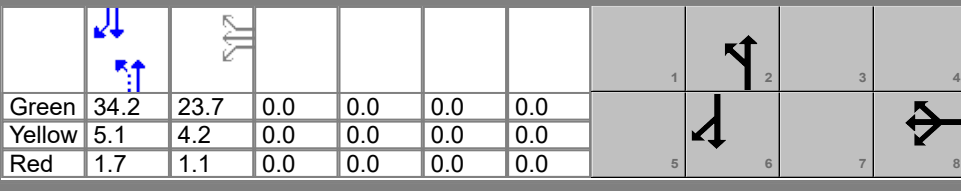
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

HCS Signalized Intersection Input Data

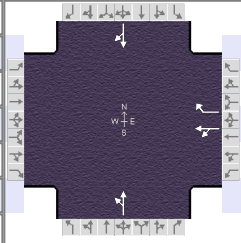
General Information						Intersection Information									
Agency	CESO					Duration, h	0.250								
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other								
Jurisdiction	ODOT		Time Period	PM Peak Hour		PHF	0.90								
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 4:30								
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD PM Peak - Int 1 & Int 2.xus											
Project Description	2043 Build Condition 1 Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h							535	1	279	157	359			329	65
Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	0	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On	Green	34.2	23.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
				Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
				Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Traffic Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h							535	1	279	157	359			329	65
Initial Queue (Q _b), veh/h							0	0	0	0	0			0	0
Base Saturation Flow Rate (s ₀), veh/h							1750	1750	1750	1750	1750			1750	1750
Parking (N _m), man/h								None			None			None	
Heavy Vehicles (P _{HV}), %								10	10		12			3	
Ped / Bike / RTOR, /h				0	0		0	0	0	0	0		0	0	0
Buses (N _b), buses/h							0	0	0	0	0	0	0	0	0
Arrival Type (AT)							3	3	3	3	3			3	3
Upstream Filtering (I)							1.00	1.00	1.00	0.87	0.87			1.00	1.00
Lane Width (W), ft								12.0	12.0		12.0			12.0	
Turn Bay Length, ft								1145	350		400			635	
Grade (P _g), %					0			0			0			0	
Speed Limit, mi/h							35	35	35	45	45			45	45
Phase Information				EBL	EBT		WBL	WBT		NBL	NBT		SBL	SBT	
Maximum Green (G _{max}) or Phase Split, s								29.0			41.0			41.0	
Yellow Change Interval (Y), s								4.2			5.1			5.1	
Red Clearance Interval (R _c), s								1.1			1.7			1.7	
Minimum Green (G _{min}), s								10			20			20	
Start-Up Lost Time (l _t), s							2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green (e), s							2.0	2.0		2.0	2.0			2.0	
Passage (PT), s								2.0			2.0			2.0	
Recall Mode								Off			Min			Min	
Dual Entry								Yes			Yes			Yes	
Walk (Walk), s					0.0			0.0			0.0			0.0	
Pedestrian Clearance Time (PC), s					0.0			0.0			0.0			0.0	
Multimodal Information				EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0	0.0	No	25.0			
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0			
Street Width / Island / Curb, ft					0		0.0	0	No	0.0	0	No	0.0		No
Width Outside / Bike Lane / Shoulder, ft							12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking				No			No	0.50		No	0.50				0.50

HCS Signalized Intersection Results Summary

General Information						Intersection Information								
Agency	CESO					Duration, h	0.250							
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other							
Jurisdiction	ODOT		Time Period	PM Peak Hour		PHF	0.90							
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 4:30							
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD PM Peak - Int 1 & Int 2.xus										
Project Description	2043 Build Condition 1 Traffic Scenario													
Demand Information			EB			WB			NB			SB		
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h						535	1	279	157	359			329	65
Signal Information														
Cycle, s	70.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On											
Force Mode	Fixed	Simult. Gap N/S	On	Green	34.2	23.7	0.0	0.0	0.0	0.0	1	2	3	4
				Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5	6	7	8
				Red	1.7	1.1	0.0	0.0	0.0	0.0				
Timer Results			EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase						8		2		6				
Case Number						11.0		8.0		8.0				
Phase Duration, s						29.0		41.0		41.0				
Change Period, (Y+R _c), s						5.3		6.8		6.8				
Max Allow Headway (MAH), s						3.2		0.0		0.0				
Queue Clearance Time (g _s), s						25.7								
Green Extension Time (g _e), s						0.0		0.0		0.0				
Phase Call Probability						1.00								
Max Out Probability						1.00								
Movement Group Results			EB			WB			NB			SB		
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement						3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h						596	310		555			438		
Adjusted Saturation Flow Rate (s), veh/h/ln						1537	1367		960			1660		
Queue Service Time (g _s), s						23.7	13.6		21.4			12.8		
Cycle Queue Clearance Time (g _c), s						23.7	13.6		34.2			12.8		
Green Ratio (g/C)						0.34	0.34		0.49			0.49		
Capacity (c), veh/h						520	463		536			811		
Volume-to-Capacity Ratio (X)						1.145	0.670		1.035			0.540		
Back of Queue (Q), ft/ln (95 th percentile)														
Back of Queue (Q), veh/ln (95 th percentile)						29.8	7.7		21.2			7.8		
Queue Storage Ratio (RQ) (95 th percentile)						0.70	0.59		1.45			0.32		
Uniform Delay (d ₁), s/veh						23.2	19.8		22.8			12.4		
Incremental Delay (d ₂), s/veh						85.8	3.0		45.5			2.6		
Initial Queue Delay (d ₃), s/veh						0.0	0.0		0.0			0.0		
Control Delay (d), s/veh						109.0	22.8		68.2			15.0		
Level of Service (LOS)						F	C		F			B		
Approach Delay, s/veh / LOS			0.0			79.5	E		68.2	E		15.0	B	
Intersection Delay, s/veh / LOS						61.3				E				
Multimodal Results			EB			WB			NB			SB		
Pedestrian LOS Score / LOS			1.71	B		1.71	B		1.66	B		1.37	A	
Bicycle LOS Score / LOS						1.98	B		1.43	A		1.21	A	

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD PM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				535	1	279	157	359			329	65

Signal Information													
Cycle, s	70.0	Reference Phase	2										
Offset, s	0	Reference Point	End										
Uncoordinated	No	Simult. Gap E/W	On	Green	34.2	23.7	0.0	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0			
				Red	1.7	1.1	0.0	0.0	0.0	0.0			

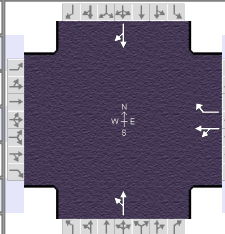
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})				0.952	0.952		0.605	0.605		1.000	0.971	
Right-Turn Adjustment Factor (f_{RT})						0.000	0.847		0.000	0.605		0.971
Left-Turn Pedestrian Adjustment Factor (f_{LPB})				1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{Rpb})						1.000			1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h				1534	3	1367	292	668	0	0	1386	274
Proportion of Vehicles Arriving on Green (P)	0.00	0.00	0.00	0.34	0.34	0.34	0.37	0.52	0.00	0.00	0.49	0.49
Incremental Delay Factor (k)					0.50	0.20		0.50			0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)				4.0		6.8		6.8
Green Ratio (g/C)				0.34		0.49		0.49
Permitted Saturation Flow Rate (s_p), veh/h/ln				0		966		1013
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						0		1709
Permitted Effective Green Time (g_p), s				0.0		34.2		0.0
Permitted Service Time (g_u), s				0.0		21.4		0.0
Permitted Queue Service Time (g_{ps}), s						21.4		
Time to First Blockage (g_t), s				0.0		3.6		34.2
Queue Service Time Before Blockage (g_{ts}), s						3.6		
Protected Right Saturation Flow (s_R), veh/h/ln				0				
Protected Right Effective Green Time (g_R), s				0.0				

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000				
Pedestrian F_s / F_{delay}	0.000	0.143	0.000	0.143	0.000	0.089	0.000	0.089				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		40.18		41.58	977.14	9.15	977.14	9.15				
Bicycle F_w / F_v	-3.64		-3.64	1.49	-3.64	0.95	-3.64	0.72				

HCS Signalized Intersection Results Graphical Summary

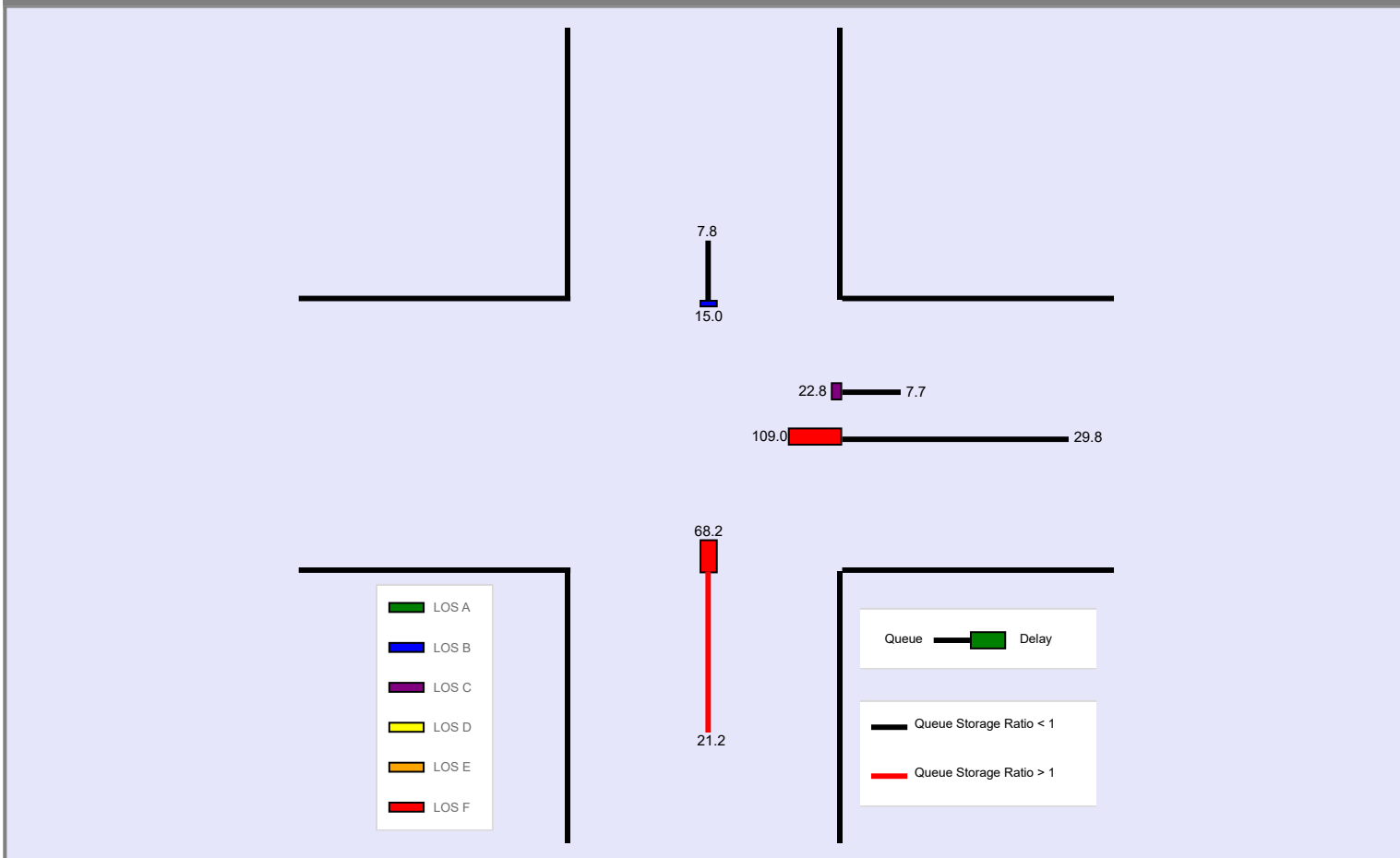
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD PM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				535	1	279	157	359			329	65

Signal Information																		
Cycle, s	70.0	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	No	Simult. Gap E/W	On	Green	34.2	23.7	0.0	0.0	0.0	0.0	1		2		3		4	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5		6		7		8	
				Red	1.7	1.1	0.0	0.0	0.0	0.0								

Movement Group Results	EB			WB			NB			SB			
	L	T	R	L	T	R	L	T	R	L	T	R	
Approach Movement													
Back of Queue (Q), ft/ln (95 th percentile)													
Back of Queue (Q), veh/ln (95 th percentile)						29.8	7.7		21.2			7.8	
Queue Storage Ratio (RQ) (95 th percentile)						0.70	0.59		1.45			0.32	
Control Delay (d), s/veh						109.0	22.8		68.2			15.0	
Level of Service (LOS)						F	C		F			B	
Approach Delay, s/veh / LOS	0.0			79.5		E			68.2		E	15.0	B
Intersection Delay, s/veh / LOS				61.3					E				



--- Messages ---

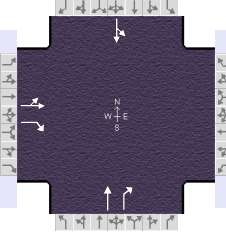
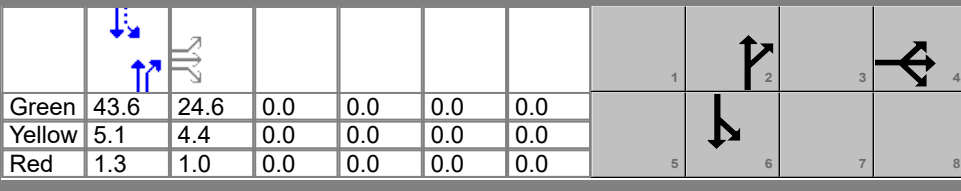
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

HCS Signalized Intersection Input Data

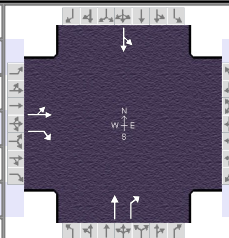
General Information						Intersection Information										
Agency	CESO					Duration, h	0.250									
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.89									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00									
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD AM Peak - Int 1 & Int 2.xus												
Project Description	2043 Build Condition 1 Traffic Scenario															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				60	3	148					430	425	127	441		
Signal Information																
Cycle, s	80.0	Reference Phase	2													
Offset, s	28	Reference Point	End													
Uncoordinated	No	Simult. Gap E/W	On													
Force Mode	Fixed	Simult. Gap N/S	On		Green	43.6	24.6	0.0	0.0	0.0	0.0					
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0							
		Red	1.3	1.0	0.0	0.0	0.0	0.0								
Traffic Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				60	3	148					430	425	127	441		
Initial Queue (Q _b), veh/h				0	0	0					0	0	0	0		
Base Saturation Flow Rate (s ₀), veh/h				1750	1750	1750					1750	1750	1750	1750		
Parking (N _m), man/h				None							None			None		
Heavy Vehicles (P _{HV}), %					19	19					13	13		16		
Ped / Bike / RTOR, /h				0	0	0	0	0		0	0	0	0	0		
Buses (N _b), buses/h				0	0	0				0	0	0	0	0		
Arrival Type (AT)				3	3	3				3	3	3	3			
Upstream Filtering (I)				1.00	1.00	1.00				1.00	1.00	0.66	0.66			
Lane Width (W), ft					12.0	12.0				12.0	12.0		12.0			
Turn Bay Length, ft					1425	250				470	410		400			
Grade (P _g), %					0			0		0			0			
Speed Limit, mi/h				35	35	35				45	45	45	45			
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s					30.0				50.0		50.0					
Yellow Change Interval (Y), s					4.4				5.1		5.1					
Red Clearance Interval (R _c), s					1.0				1.3		1.3					
Minimum Green (G _{min}), s					10				20		20					
Start-Up Lost Time (l _t), s				2.0	2.0				2.0	2.0	2.0					
Extension of Effective Green (e), s				2.0	2.0				2.0	2.0	2.0					
Passage (PT), s					2.0				2.0		2.0					
Recall Mode					Off				Min		Min					
Dual Entry					Yes				Yes		Yes					
Walk (Walk), s					0.0		0.0				0.0					
Pedestrian Clearance Time (PC), s					0.0		0.0				0.0					
Multimodal Information				EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0				0.0	No	25.0	
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0	
Street Width / Island / Curb, ft				0.0	0	No		0		0.0		No	0.0	0	No	
Width Outside / Bike Lane / Shoulder, ft				12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking				No	0.50		No				0.50		No	0.50		

HCS Signalized Intersection Results Summary

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.89									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00									
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD AM Peak - Int 1 & Int 2.xus												
Project Description	2043 Build Condition 1 Traffic Scenario															
Demand Information					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h					60	3	148				430	425		127	441	
Signal Information																
Cycle, s	80.0	Reference Phase	2													
Offset, s	28	Reference Point	End													
Uncoordinated	No	Simult. Gap E/W	On													
Force Mode	Fixed	Simult. Gap N/S	On		Green	43.6	24.6	0.0	0.0	0.0	0.0					
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0							
		Red	1.3	1.0	0.0	0.0	0.0	0.0								
Timer Results					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase						4				2		6				
Case Number						11.0				7.0		8.0				
Phase Duration, s						30.0				50.0		50.0				
Change Period, (Y+R _c), s						5.4				6.4		6.4				
Max Allow Headway (MAH), s						3.3				0.0		0.0				
Queue Clearance Time (g _s), s						10.4										
Green Extension Time (g _e), s						0.4				0.0		0.0				
Phase Call Probability						1.00										
Max Out Probability						0.00										
Movement Group Results					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement					7	4	14				2	12	1	6		
Adjusted Flow Rate (v), veh/h						71	166				483	478		701		
Adjusted Saturation Flow Rate (s), veh/h/ln						1423	1263				1573	1333		967		
Queue Service Time (g _s), s						2.9	8.4				16.1	20.3		27.5		
Cycle Queue Clearance Time (g _c), s						2.9	8.4				16.1	20.3		43.6		
Green Ratio (g/C)						0.31	0.31				0.55	0.55		0.55		
Capacity (c), veh/h						438	388				857	726		582		
Volume-to-Capacity Ratio (X)						0.162	0.428				0.564	0.657		1.205		
Back of Queue (Q), ft/ln (95 th percentile)																
Back of Queue (Q), veh/ln (95 th percentile)						1.7	4.3				8.9	9.7		39.9		
Queue Storage Ratio (RQ) (95 th percentile)						0.03	0.49				0.53	0.66		2.81		
Uniform Delay (d ₁), s/veh						20.2	22.1				12.0	12.9		29.3		
Incremental Delay (d ₂), s/veh						0.1	0.3				2.7	4.6		103.0		
Initial Queue Delay (d ₃), s/veh						0.0	0.0				0.0	0.0		0.0		
Control Delay (d), s/veh						20.3	22.4				14.6	17.5		132.3		
Level of Service (LOS)						C	C				B	B		F		
Approach Delay, s/veh / LOS					21.7	C	0.0			16.1	B	132.3	F			
Intersection Delay, s/veh / LOS					59.7					E						
Multimodal Results					EB			WB			NB			SB		
Pedestrian LOS Score / LOS					1.95	B	1.72	B	1.37	A	1.66	B				
Bicycle LOS Score / LOS					0.88	A			2.07	B	1.54	B				

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD AM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148					430	425	127	441	

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	28	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On	Green	43.6	24.6	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.4	0.0	0.0	0.0	0.0		
				Red	1.3	1.0	0.0	0.0	0.0	0.0		

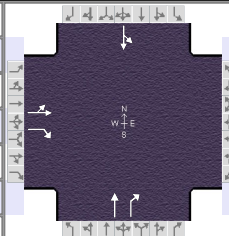
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.955	0.955					1.000	1.000		0.631	0.631	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847					0.000	0.847		0.000	0.631
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h	1355	68	1263				0	1573	1333	216	751	0
Proportion of Vehicles Arriving on Green (P)	0.31	0.31	0.31	0.00	0.00	0.00	0.00	0.55	0.55	0.57	0.35	0.00
Incremental Delay Factor (k)		0.04	0.04					0.50	0.50		0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0				6.4		6.4
Green Ratio (g/C)		0.31				0.55		0.55
Permitted Saturation Flow Rate (s_p), veh/h/ln		0				876		927
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						1573		0
Permitted Effective Green Time (g_p), s		0.0				0.0		43.6
Permitted Service Time (g_u), s		0.0				0.0		27.5
Permitted Queue Service Time (g_{ps}), s								27.5
Time to First Blockage (g_f), s		0.0				43.6		4.6
Queue Service Time Before Blockage (g_{ts}), s								4.6
Protected Right Saturation Flow (s_R), veh/h/ln		0				0		
Protected Right Effective Green Time (g_R), s		0.0				0.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	1.198	0.000	0.972	0.000	0.000	0.681	0.000	0.000	0.972	0.000		
Pedestrian F_s / F_{delay}	0.000	0.148	0.000	0.148	0.000	0.000	0.085	0.000	0.000	0.085		
Pedestrian M_{corner} / M_{cw}	0.00		0.00			0.00			0.00			
Bicycle c_b / d_b		46.66		45.16	1090.00	8.28	1090.00	8.28				
Bicycle F_w / F_v	-3.64	0.39	-3.64		-3.64	1.59	-3.64	1.05				

HCS Signalized Intersection Results Graphical Summary

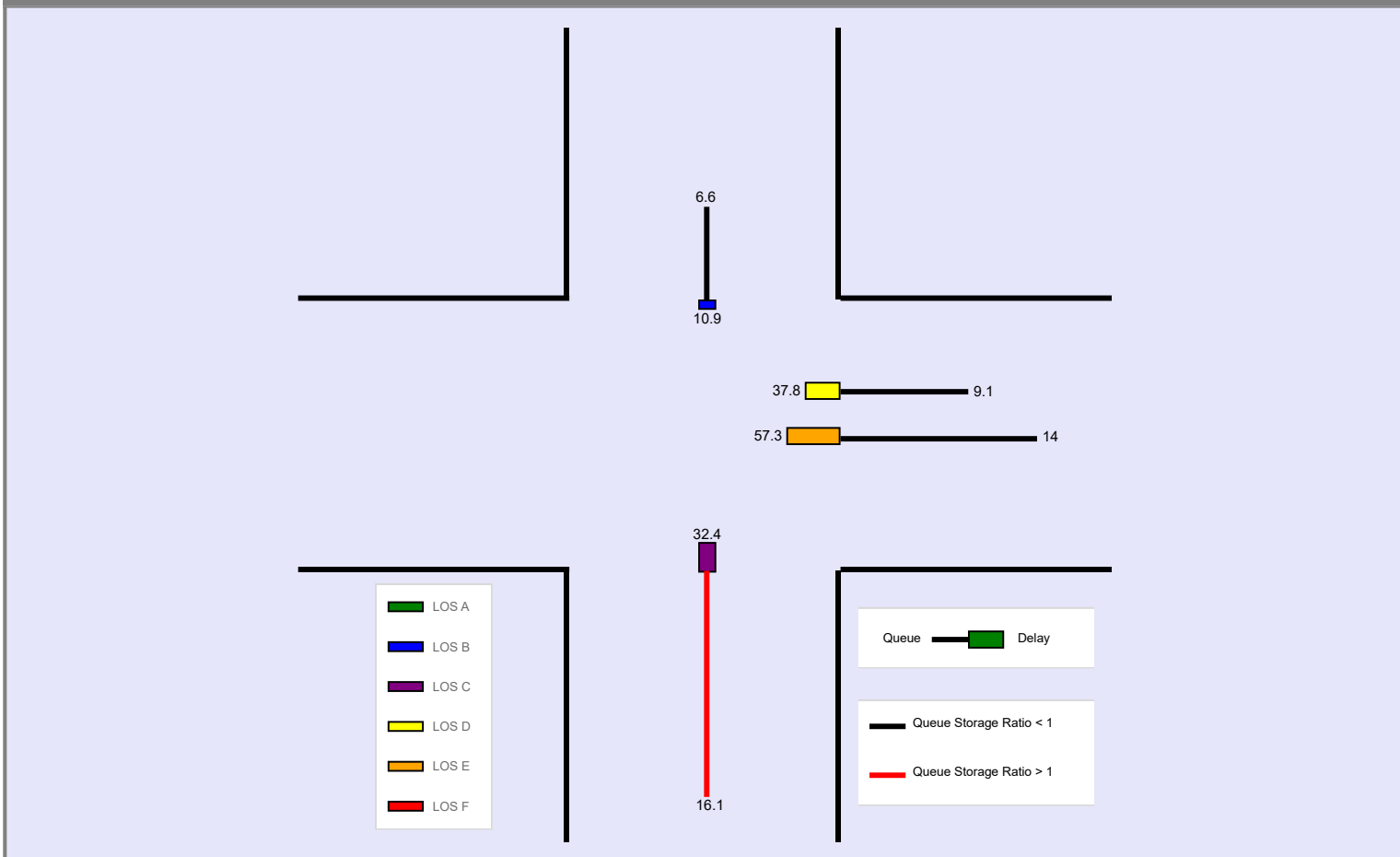
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD AM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148					430	425	127	441	

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	28	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	43.6	24.6	0.0	0.0	0.0	0.0				
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0				
		Red	1.3	1.0	0.0	0.0	0.0	0.0				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		1.7	4.3					8.9	9.7		39.9	
Queue Storage Ratio (RQ) (95 th percentile)		0.03	0.49					0.53	0.66		2.81	
Control Delay (d), s/veh		20.3	22.4					14.6	17.5		132.3	
Level of Service (LOS)		C	C					B	B		F	
Approach Delay, s/veh / LOS	21.7	C		0.0				16.1	B	132.3	F	
Intersection Delay, s/veh / LOS	59.7						E					



--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

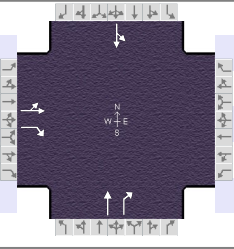
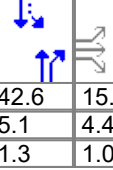
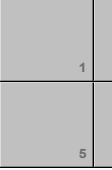
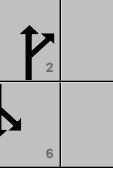
WARNING: If demand exceeds capacity, a multiple-period analysis should be conducted.

--- Comments ---

HCS Signalized Intersection Input Data

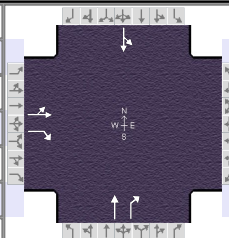
General Information						Intersection Information									
Agency	CESO					Duration, h	0.250								
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other								
Jurisdiction	ODOT		Time Period	PM Peak Hour		PHF	0.93								
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 4:30								
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD PM Peak - Int 1 & Int 2.xus											
Project Description	2043 Build Condition 1 Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				75	4	211				441	330		89	775	
Signal Information															
Cycle, s	70.0	Reference Phase	2	Green	42.6	15.6	0.0	0.0	0.0	0.0					
Offset, s	38	Reference Point	End	Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Red	1.3	1.0	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Traffic Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				75	4	211				441	330		89	775	
Initial Queue (Q _b), veh/h				0	0	0				0	0		0	0	
Base Saturation Flow Rate (s ₀), veh/h				1750	1750	1750				1750	1750		1750	1750	
Parking (N _m), man/h				None						None			None		
Heavy Vehicles (P _{HV}), %					37	37					17	17		9	
Ped / Bike / RTOR, /h				0	0	0	0	0		0	0	0	0	0	
Buses (N _b), buses/h				0	0	0				0	0	0	0	0	
Arrival Type (AT)				3	3	3				3	3		3	3	
Upstream Filtering (I)				1.00	1.00	1.00				1.00	1.00		0.28	0.28	
Lane Width (W), ft					12.0	12.0					12.0	12.0		12.0	
Turn Bay Length, ft					1425	250					470	410		400	
Grade (P _g), %					0			0			0			0	
Speed Limit, mi/h				35	35	35				45	45		45	45	
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Maximum Green (G _{max}) or Phase Split, s					21.0				49.0			49.0			
Yellow Change Interval (Y), s					4.4				5.1			5.1			
Red Clearance Interval (R _c), s					1.0				1.3			1.3			
Minimum Green (G _{min}), s					10				20			20			
Start-Up Lost Time (l _t), s				2.0	2.0				2.0	2.0	2.0				
Extension of Effective Green (e), s				2.0	2.0				2.0	2.0	2.0				
Passage (P _T), s					2.0				2.0		2.0				
Recall Mode					Off				Min		Min				
Dual Entry					Yes				Yes		Yes				
Walk (Walk), s					0.0		0.0				0.0				
Pedestrian Clearance Time (P _C), s					0.0		0.0				0.0				
Multimodal Information				EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0				0.0	No	25.0
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0
Street Width / Island / Curb, ft				0.0	0	No		0		0.0		No	0.0	0	No
Width Outside / Bike Lane / Shoulder, ft				12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking				No	0.50		No				0.50		No	0.50	

HCS Signalized Intersection Results Summary

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	JMP		Analysis Date	Sep 30, 2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	PM Peak Hour		PHF	0.93									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 4:30									
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD PM Peak - Int 1 & Int 2.xus												
Project Description	2043 Build Condition 1 Traffic Scenario															
Demand Information					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h					75	4	211				441	330		89	775	
Signal Information																
Cycle, s	70.0	Reference Phase	2		Green	42.6	15.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	38	Reference Point	End		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On		Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On													
Timer Results					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase						4				2		6				
Case Number						11.0				7.0		8.0				
Phase Duration, s						21.0				49.0		49.0				
Change Period, (Y+R _c), s						5.4				6.4		6.4				
Max Allow Headway (MAH), s						3.3				0.0		0.0				
Queue Clearance Time (g _s), s						16.9										
Green Extension Time (g _e), s						0.0				0.0		0.0				
Phase Call Probability						1.00										
Max Out Probability						1.00										
Movement Group Results					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement					7	4	14				2	12	1	6		
Adjusted Flow Rate (v), veh/h						85	227				474	355		885		
Adjusted Saturation Flow Rate (s), veh/h/ln						1189	1055				1518	1286		1455		
Queue Service Time (g _s), s						4.2	14.9				12.4	10.4		30.1		
Cycle Queue Clearance Time (g _c), s						4.2	14.9				12.4	10.4		42.6		
Green Ratio (g/C)						0.22	0.22				0.61	0.61		0.61		
Capacity (c), veh/h						265	235				924	783		942		
Volume-to-Capacity Ratio (X)						0.321	0.965				0.513	0.453		0.939		
Back of Queue (Q), ft/ln (95 th percentile)																
Back of Queue (Q), veh/ln (95 th percentile)						2.0	10.9				6.0	4.3		14.5		
Queue Storage Ratio (RQ) (95 th percentile)						0.05	1.41				0.36	0.30		0.97		
Uniform Delay (d ₁), s/veh						22.8	26.9				7.8	7.4		14.3		
Incremental Delay (d ₂), s/veh						0.3	48.4				2.0	1.9		6.8		
Initial Queue Delay (d ₃), s/veh						0.0	0.0				0.0	0.0		0.0		
Control Delay (d), s/veh						23.0	75.4				9.8	9.3		21.0		
Level of Service (LOS)						C	E				A	A		C		
Approach Delay, s/veh / LOS						61.1	E	0.0			9.6	A	21.0	C		
Intersection Delay, s/veh / LOS						22.5						C				
Multimodal Results					EB			WB			NB			SB		
Pedestrian LOS Score / LOS					1.94	B	1.71	B	1.35	A	1.64	B				
Bicycle LOS Score / LOS					1.00	A			1.86	B	2.02	B				

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1> 4:30		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD PM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	211					441	330	89	775	

Signal Information												
Cycle, s	70.0	Reference Phase	2									
Offset, s	38	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	42.6	15.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

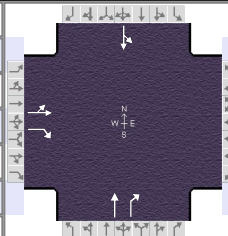
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.955	0.955					1.000	1.000		0.894	0.894	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847					0.000	0.847		0.000	0.894
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h	1128	60	1055				0	1518	1286	150	1305	0
Proportion of Vehicles Arriving on Green (P)	0.22	0.22	0.22	0.00	0.00	0.00	0.00	0.61	0.61	0.63	0.59	0.00
Incremental Delay Factor (k)		0.04	0.47					0.50	0.50		0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0				6.4		6.4
Green Ratio (g/C)		0.22				0.61		0.61
Permitted Saturation Flow Rate (s_p), veh/h/ln		0				694		934
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						1518		0
Permitted Effective Green Time (g_p), s		0.0				0.0		42.6
Permitted Service Time (g_u), s		0.0				0.0		30.2
Permitted Queue Service Time (g_{ps}), s								30.1
Time to First Blockage (g_f), s		0.0				42.6		10.4
Queue Service Time Before Blockage (g_{ts}), s								10.4
Protected Right Saturation Flow (s_R), veh/h/ln		0				0		
Protected Right Effective Green Time (g_R), s		0.0				0.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	1.198	0.000	0.972	0.000	0.681	0.000	0.972	0.000				
Pedestrian F_s / F_{delay}	0.000	0.143	0.000	0.143	0.000	0.067	0.000	0.067				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		41.69		40.18	1217.14	5.36	1217.14	5.36				
Bicycle F_w / F_v	-3.64	0.51	-3.64		-3.64	1.37	-3.64	1.53				

HCS Signalized Intersection Results Graphical Summary

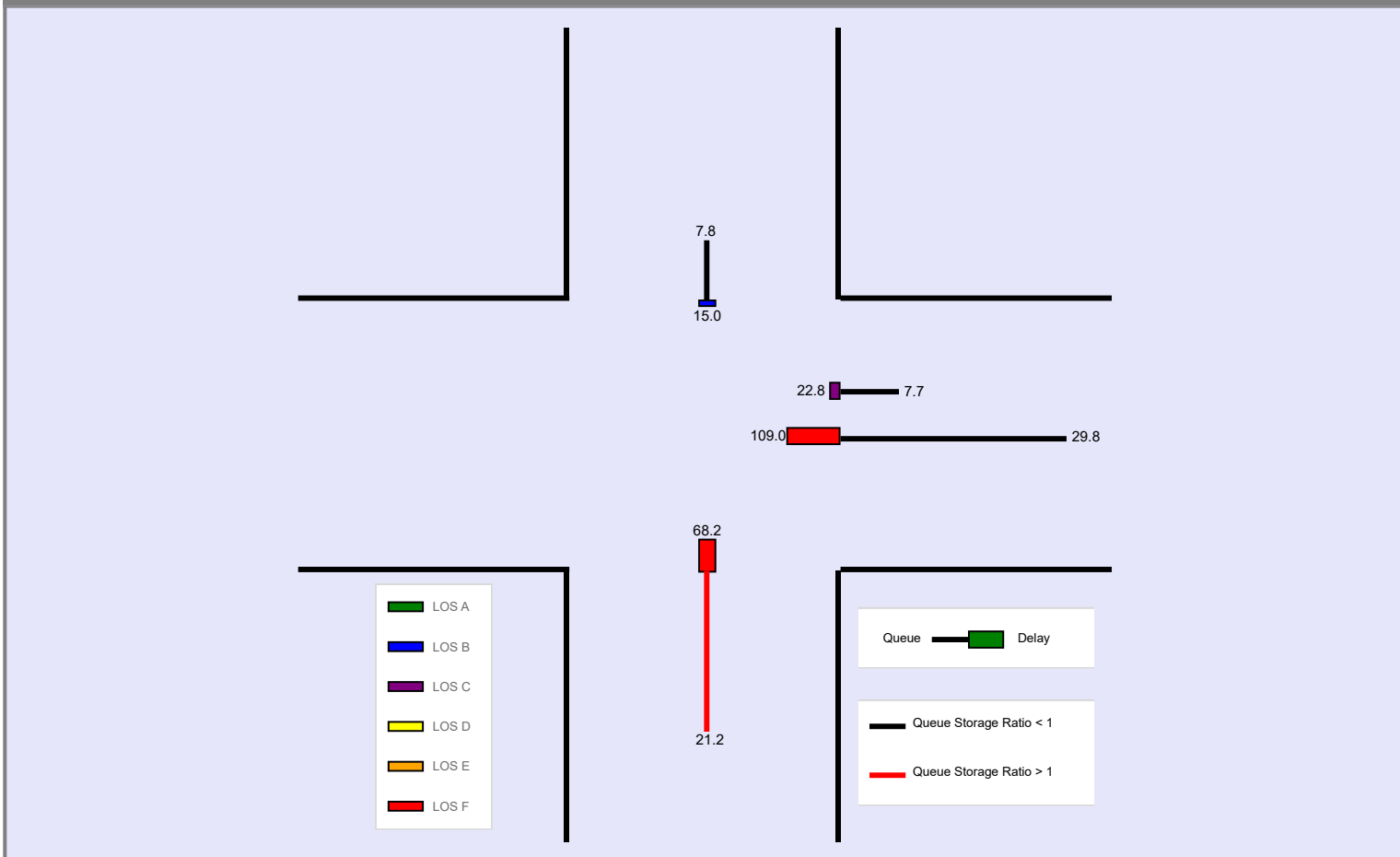
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 30, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD PM Peak - Int 1 & Int 2.xus				
Project Description	2043 Build Condition 1 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	211					441	330	89	775	

Signal Information													
Cycle, s	70.0	Reference Phase	2										
Offset, s	38	Reference Point	End										
Uncoordinated	No	Simult. Gap E/W	On										
Force Mode	Fixed	Simult. Gap N/S	On										
		Green	42.6	15.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		2.0	10.9					6.0	4.3			14.5
Queue Storage Ratio (RQ) (95 th percentile)		0.05	1.41					0.36	0.30			0.97
Control Delay (d), s/veh		23.0	75.4					9.8	9.3			21.0
Level of Service (LOS)		C	E					A	A			C
Approach Delay, s/veh / LOS	61.1		E	0.0				9.6	A	21.0		C
Intersection Delay, s/veh / LOS	22.5									C		



--- Messages ---

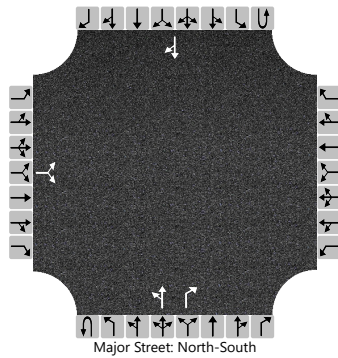
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Reco Drive		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Reco Drive		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	AM Peak Hour			Peak Hour Factor	0.90		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	1	0	0	1	0	
Configuration			LR							LT		R				TR	
Volume (veh/h)		205		107						115	284	366				351	238
Percent Heavy Vehicles (%)		0		0						15							
Proportion Time Blocked																	
Percent Grade (%)		0															
Right Turn Channelized										No							
Median Type Storage		Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.10		6.20						4.25						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.50		3.30						2.34						

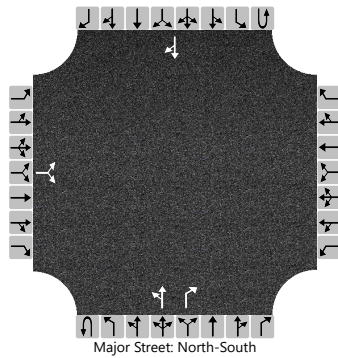
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			347							128						
Capacity, c (veh/h)			209							874						
v/c Ratio			1.66							0.15						
95% Queue Length, Q ₉₅ (veh)			22.9							0.5						
Control Delay (s/veh)			356.3							9.8	1.0					
Level of Service (LOS)			F							A	A					
Approach Delay (s/veh)		356.3								1.8						
Approach LOS		F								A						

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Reco Drive		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Reco Drive		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	PM Peak Hour			Peak Hour Factor	0.93		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound					
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R		
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6		
Number of Lanes		0	1	0		0	0	0	0	0	1	1	0	0	1	0		
Configuration			LR							LT		R				TR		
Volume (veh/h)		190		62						56	308	273				797	189	
Percent Heavy Vehicles (%)		1		1						21								
Proportion Time Blocked																		
Percent Grade (%)		0																
Right Turn Channelized										No								
Median Type Storage		Undivided																

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		7.11		6.21						4.31							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.51		3.31						2.39							

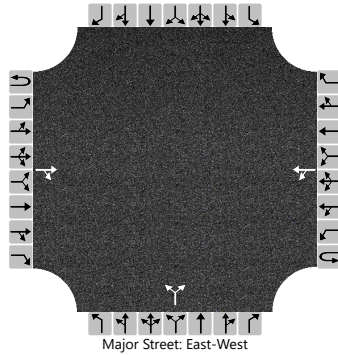
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			271							60							
Capacity, c (veh/h)			122							590							
v/c Ratio			2.22							0.10							
95% Queue Length, Q ₉₅ (veh)			23.0							0.3							
Control Delay (s/veh)			630.5							11.8	1.1						
Level of Service (LOS)			F							B	A						
Approach Delay (s/veh)		630.5								1.6							
Approach LOS		F								A							

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP	Intersection	Reco Drive & Love's Site Driveway #1				
Agency/Co.	CESO	Jurisdiction	ODOT				
Date Performed	10/18/2022	East/West Street	Reco Drive				
Analysis Year	2043	North/South Street	Love's Site Driveway #1				
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.92				
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25				
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR		LT				LR						
Volume (veh/h)			16	0		304	49			0		296				
Percent Heavy Vehicles (%)						2				0		0				
Proportion Time Blocked																
Percent Grade (%)									0							
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)						4.1				7.1		6.2				
Critical Headway (sec)						4.12				6.40		6.20				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)						2.22				3.50		3.30				

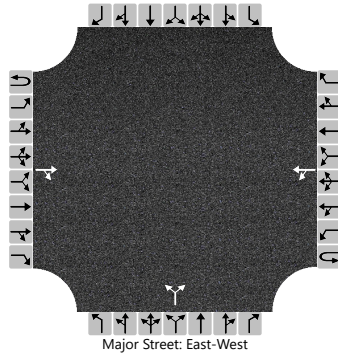
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)						330					322					
Capacity, c (veh/h)						1600					1067					
v/c Ratio						0.21					0.30					
95% Queue Length, Q ₉₅ (veh)						0.8					1.3					
Control Delay (s/veh)						7.8	1.7				9.8					
Level of Service (LOS)						A	A				A					
Approach Delay (s/veh)					7.0				9.8							
Approach LOS					A				A							

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	Reco Drive & Love's Site Driveway #1		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Reco Drive		
Analysis Year	2043			North/South Street	Love's Site Driveway #1		
Time Analyzed	PM Peak Hour			Peak Hour Factor	0.92		
Intersection Orientation	East-West			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR		LT					LR					
Volume (veh/h)			33	0		227	18			0		219				
Percent Heavy Vehicles (%)						1				0		0				
Proportion Time Blocked																
Percent Grade (%)									0							
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)						4.1				7.1		6.2				
Critical Headway (sec)						4.11				6.40		6.20				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)						2.21				3.50		3.30				

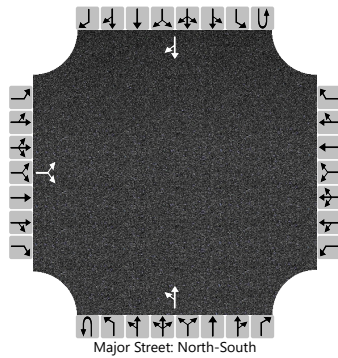
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)						247					238					
Capacity, c (veh/h)						1582					1043					
v/c Ratio						0.16					0.23					
95% Queue Length, Q ₉₅ (veh)						0.6					0.9					
Control Delay (s/veh)						7.7	1.2				9.5					
Level of Service (LOS)						A	A				A					
Approach Delay (s/veh)					7.2				9.5							
Approach LOS					A				A							

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Love's Site Driveway #2		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Love's Site Driveway #2		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	AM Peak Hour			Peak Hour Factor	0.92		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0
Configuration			LR							LT						TR
Volume (veh/h)		51		17						17	714				408	50
Percent Heavy Vehicles (%)		100		100						11						
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.40		7.20						4.21						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		4.40		4.20						2.30						

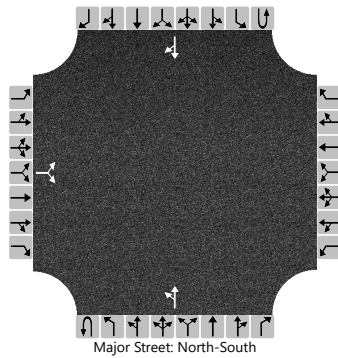
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			74							18						
Capacity, c (veh/h)			138							1021						
v/c Ratio			0.54							0.02						
95% Queue Length, Q ₉₅ (veh)			2.6							0.1						
Control Delay (s/veh)			58.0							8.6	0.3					
Level of Service (LOS)			F							A	A					
Approach Delay (s/veh)	58.0								0.5							
Approach LOS	F								A							

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Love's Site Driveway #2		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Love's Site Driveway #2		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	PM Peak Hour			Peak Hour Factor	0.92		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 1						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	0	0	0	1	0	
Configuration			LR								LT					TR	
Volume (veh/h)		59		10						12	578				794	65	
Percent Heavy Vehicles (%)		100		100						15							
Proportion Time Blocked																	
Percent Grade (%)		0															
Right Turn Channelized																	
Median Type Storage		Undivided															

Critical and Follow-up Headways

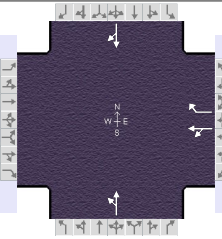
Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.40		7.20						4.25						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		4.40		4.20						2.34						

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			75							13							
Capacity, c (veh/h)			81							683							
v/c Ratio			0.93							0.02							
95% Queue Length, Q ₉₅ (veh)			5.0							0.1							
Control Delay (s/veh)			170.6							10.4	0.3						
Level of Service (LOS)			F							B	A						
Approach Delay (s/veh)		170.6								0.5							
Approach LOS		F								A							

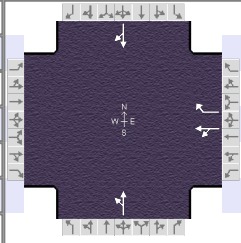
CONDITION 2

HCS Signalized Intersection Input Data

General Information					Intersection Information														
Agency	CESO				Duration, h	0.250													
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other												
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.81												
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00												
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD AM Peak - Int 1 - Int 3.xus															
Project Description	2043 Build Condition 2 Traffic Scenario																		
Demand Information					EB			WB			NB			SB					
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h								280	1	210	160	330			288	55			
Signal Information								1			2			3			4		
Cycle, s	80.0	Reference Phase	2					Green			46.2			21.7			0.0		
Offset, s	0	Reference Point	End					Yellow			5.1			4.2			0.0		
Uncoordinated	No	Simult. Gap E/W	On					Red			1.7			1.1			0.0		
Force Mode	Fixed	Simult. Gap N/S	On																
Traffic Information					EB			WB			NB			SB					
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h								280	1	210	160	330			288	55			
Initial Queue (Q _b), veh/h								0	0	0	0	0			0	0			
Base Saturation Flow Rate (s ₀), veh/h								1750	1750	1750	1750	1750			1750	1750			
Parking (N _m), man/h								None			None			None					
Heavy Vehicles (P _{HV}), %									17	17		13			11				
Ped / Bike / RTOR, /h					0	0		0	0	0	0	0		0	0	0			
Buses (N _b), buses/h								0	0	0	0	0	0	0	0	0			
Arrival Type (AT)								3	3	3	3	3		3	3				
Upstream Filtering (I)								1.00	1.00	1.00	0.97	0.97		1.00	1.00				
Lane Width (W), ft									12.0	12.0		12.0		12.0					
Turn Bay Length, ft									1145	350		400		635					
Grade (P _g), %						0			0			0		0					
Speed Limit, mi/h								35	35	35	45	45		45	45				
Phase Information					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT							
Maximum Green (G _{max}) or Phase Split, s								52.0		28.0		28.0							
Yellow Change Interval (Y), s								4.2		5.1		5.1							
Red Clearance Interval (R _c), s								1.1		1.7		1.7							
Minimum Green (G _{min}), s								10		20		20							
Start-Up Lost Time (l _t), s							2.0	2.0	2.0	2.0		2.0							
Extension of Effective Green (e), s							2.0	2.0	2.0	2.0		2.0							
Passage (PT), s								2.0		2.0		2.0							
Recall Mode								Off		Min		Min							
Dual Entry								Yes		Yes		Yes							
Walk (Walk), s						0.0		0.0		0.0		0.0							
Pedestrian Clearance Time (PC), s						0.0		0.0		0.0		0.0							
Multimodal Information					EB			WB			NB			SB					
85th % Speed / Rest in Walk / Corner Radius					0.0	No	25.0	0.0	No	25.0	0.0	No	25.0						
Walkway / Crosswalk Width / Length, ft					9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0						
Street Width / Island / Curb, ft						0		0.0	0	No	0.0	0	No	0.0		No			
Width Outside / Bike Lane / Shoulder, ft								12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0			
Pedestrian Signal / Occupied Parking					No			No	0.50		No	0.50			0.50				

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				280	1	210	160	330			288	55

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On	Green	46.2	21.7	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0		
				Red	1.7	1.1	0.0	0.0	0.0	0.0		

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				8		2		6
Case Number				11.0		8.0		8.0
Phase Duration, s				27.0		53.0		53.0
Change Period, (Y+R _c), s				5.3		6.8		6.8
Max Allow Headway (MAH), s				3.2		0.0		0.0
Queue Clearance Time (g _s), s				20.4				
Green Extension Time (g _e), s				1.3		0.0		0.0
Phase Call Probability				1.00				
Max Out Probability				0.00				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h					347	259		341				423
Adjusted Saturation Flow Rate (s), veh/h/ln					1446	1286		1049				1555
Queue Service Time (g _s), s					18.4	14.7		9.6				12.6
Cycle Queue Clearance Time (g _c), s					18.4	14.7		22.2				12.6
Green Ratio (g/C)					0.27	0.27		0.58				0.58
Capacity (c), veh/h					392	349		666				898
Volume-to-Capacity Ratio (X)					0.885	0.743		0.512				0.471
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)					10.4	7.8		6.8				6.9
Queue Storage Ratio (RQ) (95 th percentile)					0.26	0.63		0.47				0.30
Uniform Delay (d ₁), s/veh					28.0	26.6		11.3				9.8
Incremental Delay (d ₂), s/veh					2.7	1.2		2.7				1.8
Initial Queue Delay (d ₃), s/veh					0.0	0.0		0.0				0.0
Control Delay (d), s/veh					30.7	27.8		14.0				11.6
Level of Service (LOS)					C	C		B				B
Approach Delay, s/veh / LOS	0.0			29.4		C	14.0		B	11.6		B
Intersection Delay, s/veh / LOS				20.1						C		

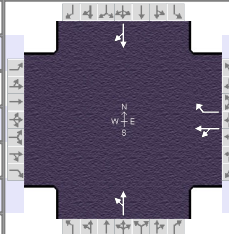
Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.72	B	1.72	B	1.65	B	1.36	A
Bicycle LOS Score / LOS			1.49	A	1.49	A	1.19	A

HCS Signalized Intersection Intermediate Values

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	TMC		Analysis Date	11/18/2022	Area Type	Other										
Jurisdiction	ODOT		Time Period	AM Peak Hour	PHF	0.81										
Urban Street	SR-149		Analysis Year	2043	Analysis Period	1 > 7:00										
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD AM Peak - Int 1 - Int 3.xus												
Project Description	2043 Build Condition 2 Traffic Scenario															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h							280	1	210	160	330			288	55	
Signal Information																
Cycle, s	80.0	Reference Phase	2	Green	46.2	21.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Offset, s	0	Reference Point	End	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Uncoordinated	No	Simult. Gap E/W	On	Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On													
Saturation Flow / Delay				L	T	R	L	T	R	L	T	R	L	T	R	
Lane Width Adjustment Factor (f _w)																
Heavy Vehicles and Grade Factor (f _{HVg})																
Parking Activity Adjustment Factor (f _p)				0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f _{bb})				0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f _a)																
Lane Utilization Adjustment Factor (f _{LU})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f _{LT})							0.953	0.953		0.667	0.667		1.000	0.972		
Right-Turn Adjustment Factor (f _{RT})									0.000	0.847		0.000	0.667		0.000	0.972
Left-Turn Pedestrian Adjustment Factor (f _{LPB})							1.000			1.000			1.000			
Right-Turn Ped-Bike Adjustment Factor (f _{RPB})									1.000			1.000				1.000
Work Zone Adjustment Factor (f _{wz})																
DDI Factor (f _{DDI})																
Movement Saturation Flow Rate (s), veh/h							1441	5	1286	342	706	0	0	1306	249	
Proportion of Vehicles Arriving on Green (P)				0.00	0.00	0.00	0.27	0.27	0.27	0.49	0.68	0.00	0.00	0.58	0.58	
Incremental Delay Factor (k)								0.04	0.04		0.50			0.50		
Signal Timing / Movement Groups				EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R					
Lost Time (t _L)							4.0		6.8		6.8					
Green Ratio (g/C)							0.27		0.58		0.58					
Permitted Saturation Flow Rate (s _p), veh/h/ln							0		979		1169					
Shared Saturation Flow Rate (s _{sh}), veh/h/ln									0		1600					
Permitted Effective Green Time (g _p), s							0.0		46.2		0.0					
Permitted Service Time (g _u), s							0.0		33.6		0.0					
Permitted Queue Service Time (g _{ps}), s									9.6							
Time to First Blockage (g _t), s							0.0		4.1		46.2					
Queue Service Time Before Blockage (g _{ts}), s									4.1							
Protected Right Saturation Flow (s _R), veh/h/ln							0									
Protected Right Effective Green Time (g _R), s							0.0									
Multimodal				EB			WB			NB			SB			
Pedestrian F _w / F _v				0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000					
Pedestrian F _s / F _{delay}				0.000	0.148	0.000	0.148	0.000	0.079	0.000	0.079					
Pedestrian M _{corner} / M _{cw}				0.00		0.00		0.00		0.00						
Bicycle c _b / d _b					45.16		46.55	1155.28	7.14	1155.28	7.14					
Bicycle F _w / F _v				-3.64		-3.64	1.00	-3.64	1.00	-3.64	0.70					

HCS Signalized Intersection Results Graphical Summary

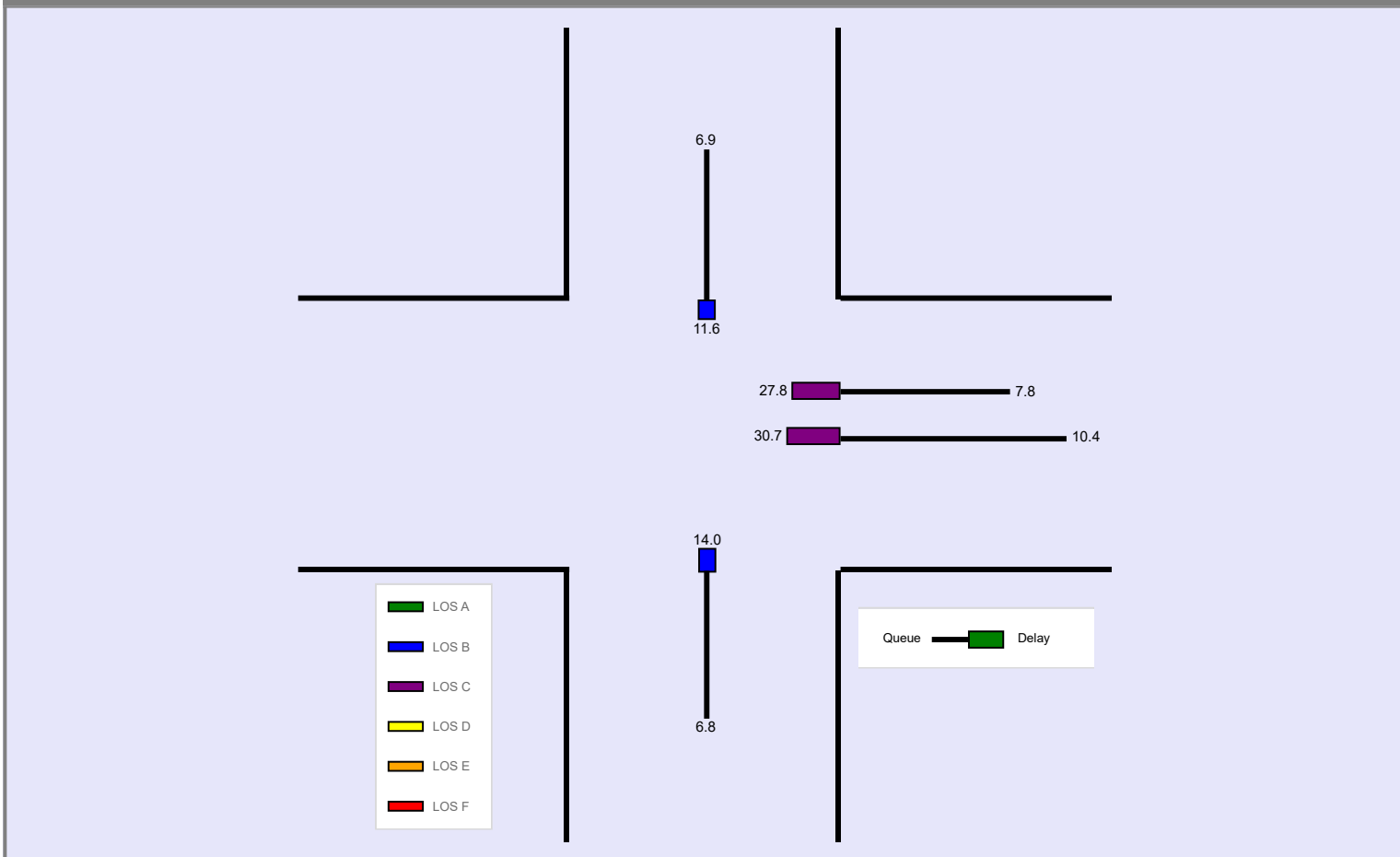
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				280	1	210	160	330			288	55

Signal Information											
Cycle, s	80.0	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	No	Simult. Gap E/W	On	Green	46.2	21.7	0.0	0.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	
				Red	1.7	1.1	0.0	0.0	0.0	0.0	

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)						10.4	7.8		6.8			6.9
Queue Storage Ratio (RQ) (95 th percentile)						0.26	0.63		0.47			0.30
Control Delay (d), s/veh						30.7	27.8		14.0			11.6
Level of Service (LOS)						C	C		B			B
Approach Delay, s/veh / LOS	0.0			29.4		C		14.0	B		11.6	B
Intersection Delay, s/veh / LOS	20.1						C					

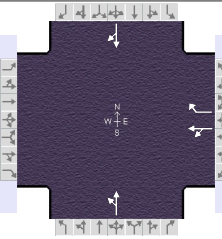


--- Messages ---

WARNING: According to input data, upstream feeding volume is equal to 56% of downstream exit volume during time period #1, for thru movement #2.

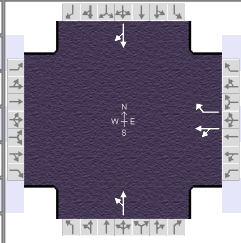
--- Comments ---

HCS Signalized Intersection Input Data

General Information						Intersection Information												
Agency	CESO					Duration, h	0.250											
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other											
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.90											
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00											
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 BD PM Peak - Int 1 - Int 3.xus														
Project Description	2043 Build Condition 2 Traffic Scenario																	
Demand Information				EB			WB			NB			SB					
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h							535	1	279	157	359			329	65			
Signal Information							1			2			3			4		
Cycle, s	70.0	Reference Phase	2				Green	29.3	28.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End				Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On				Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On															
Traffic Information				EB			WB			NB			SB					
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R			
Demand (v), veh/h							535	1	279	157	359			329	65			
Initial Queue (Q _b), veh/h							0	0	0	0	0			0	0			
Base Saturation Flow Rate (s ₀), veh/h							1750	1750	1750	1750	1750			1750	1750			
Parking (N _m), man/h								None			None			None				
Heavy Vehicles (P _{HV}), %								10	10		12			3				
Ped / Bike / RTOR, /h				0	0		0	0	0	0	0		0	0	0			
Buses (N _b), buses/h							0	0	0	0	0	0	0	0	0			
Arrival Type (AT)							3	3	3	3	3			3	3			
Upstream Filtering (I)							1.00	1.00	1.00	0.95	0.95			1.00	1.00			
Lane Width (W), ft								12.0	12.0		12.0			12.0				
Turn Bay Length, ft								1145	350		400			635				
Grade (P _g), %					0			0			0			0				
Speed Limit, mi/h							35	35	35	45	45			45	45			
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT							
Maximum Green (G _{max}) or Phase Split, s							35.0		35.0		35.0							
Yellow Change Interval (Y), s							4.2		5.1		5.1							
Red Clearance Interval (R _c), s							1.1		1.7		1.7							
Minimum Green (G _{min}), s							10		20		20							
Start-Up Lost Time (l _t), s						2.0	2.0	2.0	2.0		2.0							
Extension of Effective Green (e), s						2.0	2.0	2.0	2.0		2.0							
Passage (PT), s							2.0		2.0		2.0							
Recall Mode							Off		Min		Min							
Dual Entry							Yes		Yes		Yes							
Walk (Walk), s					0.0		0.0		0.0		0.0							
Pedestrian Clearance Time (PC), s					0.0		0.0		0.0		0.0							
Multimodal Information				EB			WB			NB			SB					
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0	0.0	No	25.0						
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0						
Street Width / Island / Curb, ft					0		0.0	0	No	0.0	0	No	0.0		No			
Width Outside / Bike Lane / Shoulder, ft							12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0			
Pedestrian Signal / Occupied Parking				No			No	0.50		No	0.50			0.50				

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				535	1	279	157	359			329	65

Signal Information														
Cycle, s	70.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	29.3	28.6	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0				
				Red	1.7	1.1	0.0	0.0	0.0	0.0				

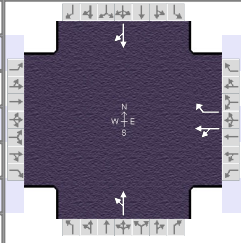
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				8		2		6
Case Number				11.0		8.0		8.0
Phase Duration, s				33.9		36.1		36.1
Change Period, (Y+R _c), s				5.3		6.8		6.8
Max Allow Headway (MAH), s				3.2		0.0		0.0
Queue Clearance Time (g _s), s				28.2				
Green Extension Time (g _e), s				0.5		0.0		0.0
Phase Call Probability				1.00				
Max Out Probability				1.00				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h				596	310		387				438	
Adjusted Saturation Flow Rate (s), veh/h/ln				1537	1367		884				1660	
Queue Service Time (g _s), s				26.2	12.1		14.7				14.6	
Cycle Queue Clearance Time (g _c), s				26.2	12.1		29.3				14.6	
Green Ratio (g/C)				0.41	0.41		0.42				0.42	
Capacity (c), veh/h				629	560		437				694	
Volume-to-Capacity Ratio (X)				0.947	0.554		0.886				0.631	
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)					17.7	6.2		13.1			9.3	
Queue Storage Ratio (RQ) (95 th percentile)					0.42	0.48		0.89			0.37	
Uniform Delay (d ₁), s/veh					19.9	15.8		23.7			16.1	
Incremental Delay (d ₂), s/veh					22.2	0.6		21.5			4.3	
Initial Queue Delay (d ₃), s/veh					0.0	0.0		0.0			0.0	
Control Delay (d), s/veh					42.2	16.4		45.2			20.4	
Level of Service (LOS)					D	B		D			C	
Approach Delay, s/veh / LOS	0.0			33.3		C	45.2		D	20.4		C
Intersection Delay, s/veh / LOS				32.7				C				

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.71	B	1.71	B	1.67	B	1.38	A
Bicycle LOS Score / LOS			1.98	B	1.43	A	1.21	A

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				535	1	279	157	359			329	65

Signal Information				Signal Phases										
Cycle, s	70.0	Reference Phase	2	↓	↑	↔					1	2	3	4
Offset, s	0	Reference Point	End	↕	↕	↕					5	6	7	8
Uncoordinated	No	Simult. Gap E/W	On	Green	29.3	28.6	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0				
				Red	1.7	1.1	0.0	0.0	0.0	0.0				

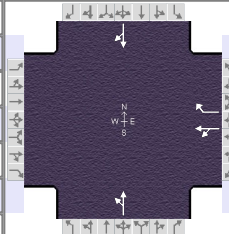
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})				0.952	0.952		0.557	0.557		1.000	0.971	
Right-Turn Adjustment Factor (f_{RT})						0.000	0.847		0.000	0.557		0.971
Left-Turn Pedestrian Adjustment Factor (f_{LPB})				1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})						1.000			1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h				1534	3	1367	269	615	0	0	1386	274
Proportion of Vehicles Arriving on Green (P)	0.00	0.00	0.00	0.41	0.41	0.41	0.58	0.31	0.00	0.00	0.42	0.42
Incremental Delay Factor (k)					0.42	0.07		0.50			0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)				4.0		6.8		6.8
Green Ratio (g/C)				0.41		0.42		0.42
Permitted Saturation Flow Rate (s_p), veh/h/ln				0		966		1128
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						0		1709
Permitted Effective Green Time (g_p), s				0.0		29.3		0.0
Permitted Service Time (g_u), s				0.0		14.7		0.0
Permitted Queue Service Time (g_{ps}), s						14.7		
Time to First Blockage (g_t), s				0.0		4.6		29.3
Queue Service Time Before Blockage (g_{ts}), s						4.6		
Protected Right Saturation Flow (s_R), veh/h/ln				0				
Protected Right Effective Green Time (g_R), s				0.0				

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000				
Pedestrian F_s / F_{delay}	0.000	0.143	0.000	0.143	0.000	0.099	0.000	0.099				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		40.18		41.58	835.84	11.86	835.84	11.86				
Bicycle F_w / F_v	-3.64		-3.64	1.49	-3.64	0.95	-3.64	0.72				

HCS Signalized Intersection Results Graphical Summary

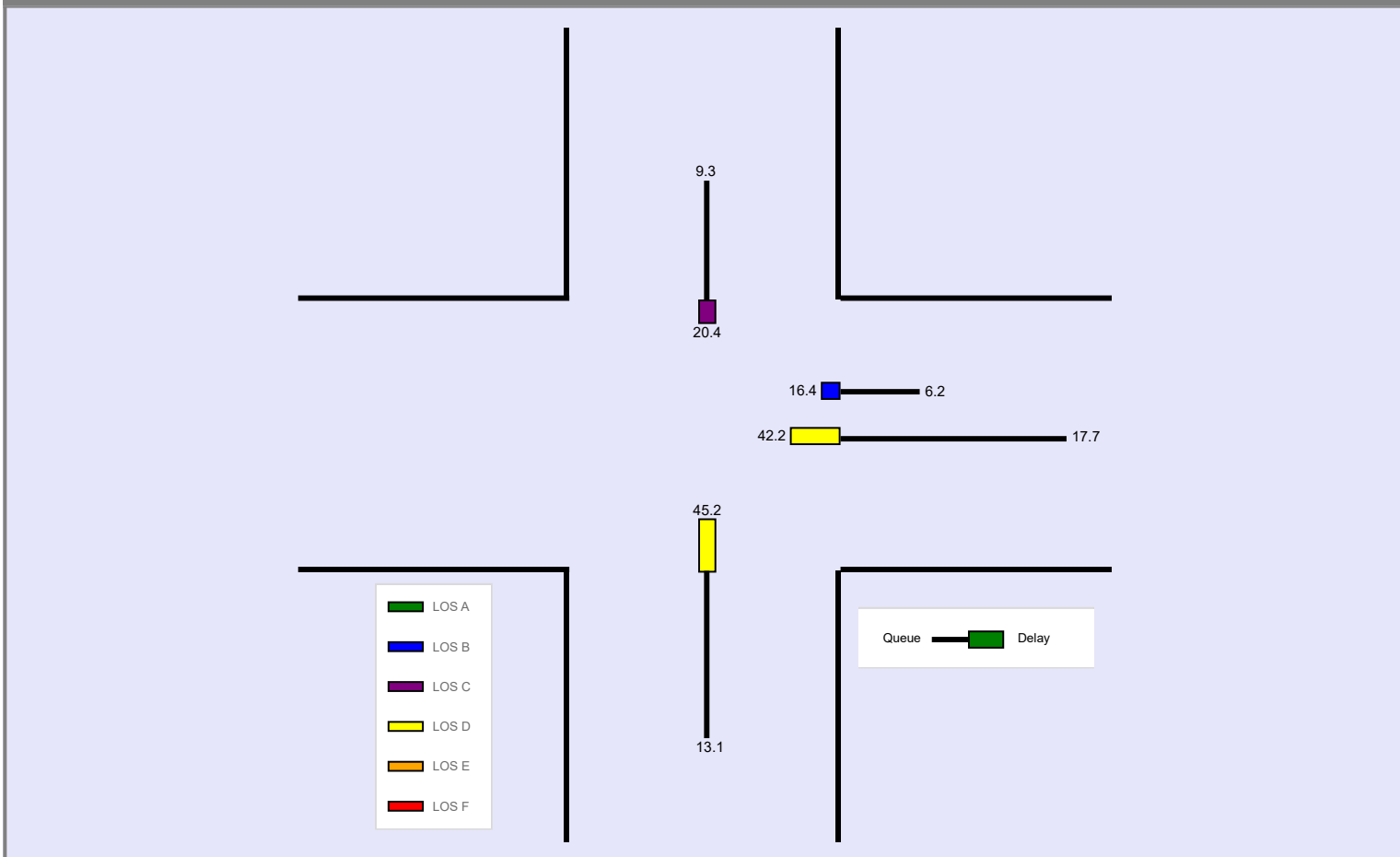
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				535	1	279	157	359			329	65

Signal Information				Signal Phases										
Cycle, s	70.0	Reference Phase	2	↓	↑	↔	↔	↔	↔	↔	↔	↔	↔	
Offset, s	0	Reference Point	End	Green	29.3	28.6	0.0	0.0	0.0	0.0	1	2	3	4
Uncoordinated	No	Simult. Gap E/W	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5	6	7	8
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.7	1.1	0.0	0.0	0.0	0.0	5	6	7	8

Movement Group Results	EB			WB			NB			SB				
	L	T	R	L	T	R	L	T	R	L	T	R		
Approach Movement														
Back of Queue (Q), ft/ln (95 th percentile)														
Back of Queue (Q), veh/ln (95 th percentile)						17.7	6.2		13.1			9.3		
Queue Storage Ratio (RQ) (95 th percentile)						0.42	0.48		0.89			0.37		
Control Delay (d), s/veh						42.2	16.4		45.2			20.4		
Level of Service (LOS)						D	B		D			C		
Approach Delay, s/veh / LOS	0.0			33.3		C		45.2	D		20.4	C		
Intersection Delay, s/veh / LOS							32.7							C



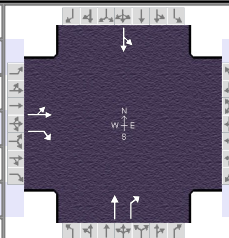
--- Messages ---

WARNING: According to input data, upstream feeding volume is equal to 67% of downstream exit volume during time period #1, for thru movement #2.

--- Comments ---

HCS Signalized Intersection Input Data

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148					430	425	127	441	

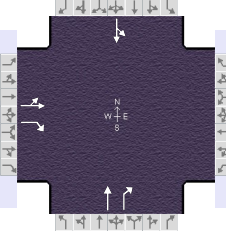
Signal Information															
Cycle, s	80.0	Reference Phase	2												
Offset, s	21	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On	Green	55.6	12.6	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
				Red	1.3	1.0	0.0	0.0	0.0	0.0					

Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148					430	425	127	441	
Initial Queue (Q _b), veh/h	0	0	0					0	0	0	0	
Base Saturation Flow Rate (s _o), veh/h	1750	1750	1750					1750	1750	1750	1750	
Parking (N _m), man/h		None						None			None	
Heavy Vehicles (P _{HV}), %		19	19					13	13		16	
Ped / Bike / RTOR, /h	0	0	0	0	0			0	0	0	0	
Buses (N _b), buses/h	0	0	0					0	0	0	0	0
Arrival Type (AT)	3	3	3					3	3	3	3	
Upstream Filtering (I)	1.00	1.00	1.00					0.79	0.79	0.68	0.68	
Lane Width (W), ft		12.0	12.0					12.0	12.0		12.0	
Turn Bay Length, ft		1425	250					250	250		400	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	35	35	35					45	45	45	45	

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Maximum Green (G _{max}) or Phase Split, s		44.0				36.0		36.0
Yellow Change Interval (Y), s		4.4				5.1		5.1
Red Clearance Interval (R _c), s		1.0				1.3		1.3
Minimum Green (G _{min}), s		10				20		20
Start-Up Lost Time (l _t), s	2.0	2.0				2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0				2.0	2.0	2.0
Passage (PT), s		2.0				2.0		2.0
Recall Mode		Off				Min		Min
Dual Entry		Yes				Yes		Yes
Walk (Walk), s		0.0		0.0				0.0
Pedestrian Clearance Time (PC), s		0.0		0.0				0.0

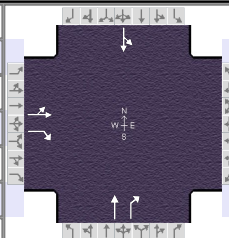
Multimodal Information	EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius	0.0	No	25.0	0.0	No	25.0				0.0	No	25.0
Walkway / Crosswalk Width / Length, ft	9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0
Street Width / Island / Curb, ft	0.0	0	No		0		0.0		No	0.0	0	No
Width Outside / Bike Lane / Shoulder, ft	12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking	No		0.50	No					0.50	No		0.50

HCS Signalized Intersection Results Summary

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.89									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00									
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD AM Peak - Int 1 - Int 3.xus												
Project Description	2043 Build Condition 2 Traffic Scenario															
Demand Information					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h					60	3	148				430	425		127	441	
Signal Information																
Cycle, s	80.0	Reference Phase	2													
Offset, s	21	Reference Point	End													
Uncoordinated	No	Simult. Gap E/W	On													
Force Mode	Fixed	Simult. Gap N/S	On		Green	55.6	12.6	0.0	0.0	0.0	0.0					
		Yellow	5.1		4.4	0.0	0.0	0.0	0.0							
		Red	1.3		1.0	0.0	0.0	0.0	0.0							
Timer Results					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase						4				2		6				
Case Number						11.0				7.0		8.0				
Phase Duration, s						18.0				62.0		62.0				
Change Period, (Y+R _c), s						5.4				6.4		6.4				
Max Allow Headway (MAH), s						3.3				0.0		0.0				
Queue Clearance Time (g _s), s						12.2										
Green Extension Time (g _e), s						0.5				0.0		0.0				
Phase Call Probability						0.99										
Max Out Probability						0.00										
Movement Group Results					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement					7	4	14				2	12	1	6		
Adjusted Flow Rate (v), veh/h						71	166				273	270		701		
Adjusted Saturation Flow Rate (s), veh/h/ln						1423	1263				1573	1333		1341		
Queue Service Time (g _s), s						3.5	10.2				8.1	6.4		28.0		
Cycle Queue Clearance Time (g _c), s						3.5	10.2				8.1	6.4		36.1		
Green Ratio (g/C)						0.16	0.16				0.69	0.69		0.69		
Capacity (c), veh/h						225	199				1092	926		986		
Volume-to-Capacity Ratio (X)						0.315	0.834				0.250	0.292		0.711		
Back of Queue (Q), ft/ln (95 th percentile)																
Back of Queue (Q), veh/ln (95 th percentile)						2.1	5.7				4.0	2.4		16.6		
Queue Storage Ratio (RQ) (95 th percentile)						0.04	0.66				0.44	0.26		1.17		
Uniform Delay (d ₁), s/veh						29.9	32.7				8.3	4.9		17.9		
Incremental Delay (d ₂), s/veh						0.3	3.5				0.4	0.6		3.0		
Initial Queue Delay (d ₃), s/veh						0.0	0.0				0.0	0.0		0.0		
Control Delay (d), s/veh						30.1	36.1				8.7	5.5		20.9		
Level of Service (LOS)						C	D				A	A		C		
Approach Delay, s/veh / LOS					34.4	C	0.0			7.1	A	20.9	C			
Intersection Delay, s/veh / LOS					18.0			B								
Multimodal Results					EB			WB			NB			SB		
Pedestrian LOS Score / LOS					1.95	B	1.72	B	1.33	A	1.62	B				
Bicycle LOS Score / LOS					0.88	A			2.07	B	1.54	B				

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148					430	425	127	441	

Signal Information																	
Cycle, s	80.0	Reference Phase	2														
Offset, s	21	Reference Point	End	Green	55.6	12.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On	Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On	Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

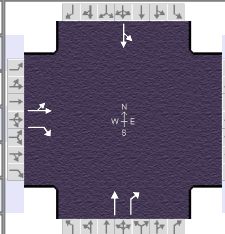
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R	
Lane Width Adjustment Factor (f_w)													
Heavy Vehicles and Grade Factor (f_{HVg})													
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	
Area Type Adjustment Factor (f_a)													
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Left-Turn Adjustment Factor (f_{LT})	0.955	0.955					1.000	1.000		0.875	0.875		
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847						0.000	0.847		0.000	0.875
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000			
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000				1.000
Work Zone Adjustment Factor (f_{wz})													
DDI Factor (f_{DDI})													
Movement Saturation Flow Rate (s), veh/h	1355	68	1263				0	1573	1333	300	1041	0	
Proportion of Vehicles Arriving on Green (P)	0.16	0.16	0.16	0.00	0.00	0.00	0.00	0.49	0.68	0.72	0.33	0.00	
Incremental Delay Factor (k)		0.04	0.04					0.50	0.50		0.50		

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0				6.4		6.4
Green Ratio (g/C)		0.16				0.69		0.69
Permitted Saturation Flow Rate (s_p), veh/h/ln		0				876		1124
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						1573		0
Permitted Effective Green Time (g_p), s		0.0				0.0		55.6
Permitted Service Time (g_u), s		0.0				0.0		47.5
Permitted Queue Service Time (g_{ps}), s								28.0
Time to First Blockage (g_f), s		0.0				55.6		6.4
Queue Service Time Before Blockage (g_{ts}), s								6.4
Protected Right Saturation Flow (s_R), veh/h/ln		0				0		
Protected Right Effective Green Time (g_R), s		0.0				0.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	1.198	0.000	0.972	0.000	0.681	0.000	0.972	0.000				
Pedestrian F_s / F_{delay}	0.000	0.148	0.000	0.148	0.000	0.053	0.000	0.053				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		46.66		45.16	1389.27	3.73	1389.27	3.73				
Bicycle F_w / F_v	-3.64	0.39	-3.64		-3.64	1.59	-3.64	1.05				

HCS Signalized Intersection Results Graphical Summary

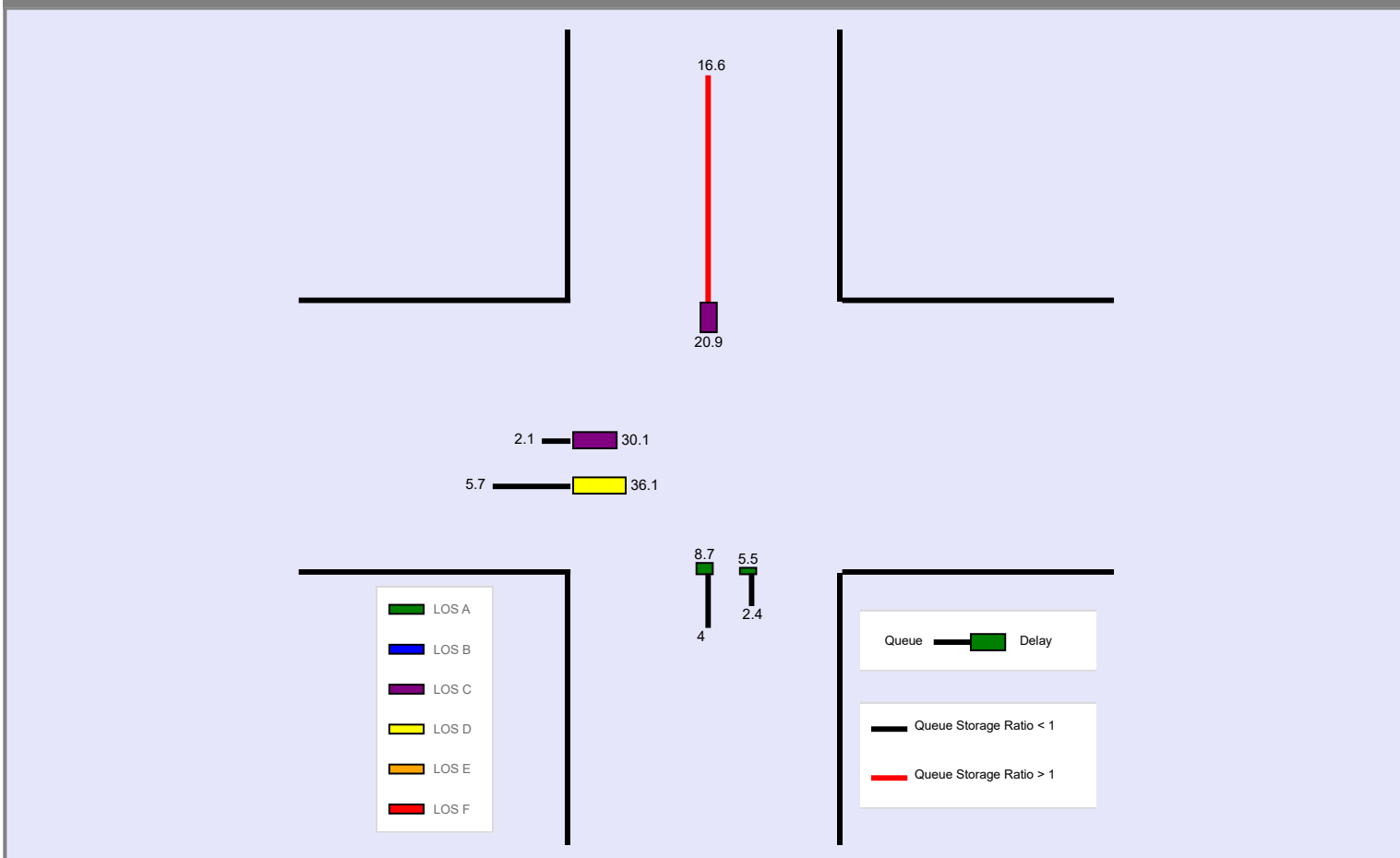
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	148						430	425	127	441

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	21	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	55.6	12.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		2.1	5.7					4.0	2.4			16.6
Queue Storage Ratio (RQ) (95 th percentile)		0.04	0.66					0.44	0.26			1.17
Control Delay (d), s/veh		30.1	36.1					8.7	5.5			20.9
Level of Service (LOS)		C	D					A	A			C
Approach Delay, s/veh / LOS	34.4	C		0.0			7.1	A		20.9	C	
Intersection Delay, s/veh / LOS	18.0						B					



--- Messages ---

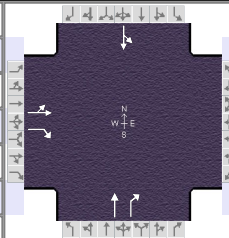
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: According to input data, upstream feeding volume is equal to 57% of downstream exit volume during time period #1, for thru movement #2.

--- Comments ---

HCS Signalized Intersection Input Data

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	211					441	330	89	775	

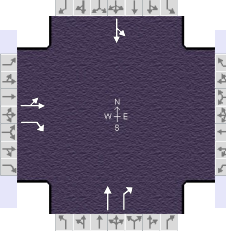
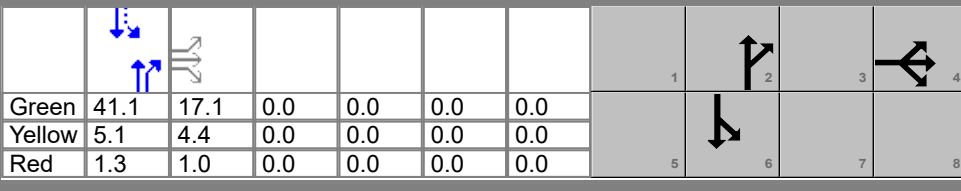
Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	59	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On	Green	41.1	17.1	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
				Red	1.3	1.0	0.0	0.0	0.0	0.0					

Traffic Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	211					441	330	89	775	
Initial Queue (Q _b), veh/h	0	0	0					0	0	0	0	
Base Saturation Flow Rate (s ₀), veh/h	1750	1750	1750					1750	1750	1750	1750	
Parking (N _m), man/h		None						None			None	
Heavy Vehicles (P _{HV}), %		37	37					17	17		9	
Ped / Bike / RTOR, /h	0	0	0	0	0			0	0	0	0	0
Buses (N _b), buses/h	0	0	0					0	0	0	0	0
Arrival Type (AT)	3	3	3					3	3	3	3	
Upstream Filtering (I)	1.00	1.00	1.00					0.81	0.81	0.45	0.45	
Lane Width (W), ft		12.0	12.0					12.0	12.0		12.0	
Turn Bay Length, ft		1425	250					250	250		400	
Grade (P _g), %		0			0			0			0	
Speed Limit, mi/h	35	35	35					45	45	45	45	

Phase Information	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Maximum Green (G _{max}) or Phase Split, s		37.0				33.0		33.0
Yellow Change Interval (Y), s		4.4				5.1		5.1
Red Clearance Interval (R _c), s		1.0				1.3		1.3
Minimum Green (G _{min}), s		10				20		20
Start-Up Lost Time (lt), s	2.0	2.0				2.0	2.0	2.0
Extension of Effective Green (e), s	2.0	2.0				2.0	2.0	2.0
Passage (PT), s		2.0				2.0		2.0
Recall Mode		Off				Min		Min
Dual Entry		Yes				Yes		Yes
Walk (Walk), s		0.0		0.0				0.0
Pedestrian Clearance Time (PC), s		0.0		0.0				0.0

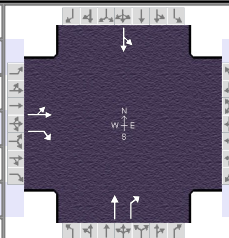
Multimodal Information	EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius	0.0	No	25.0	0.0	No	25.0				0.0	No	25.0
Walkway / Crosswalk Width / Length, ft	9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0
Street Width / Island / Curb, ft	0.0	0	No		0		0.0		No	0.0	0	No
Width Outside / Bike Lane / Shoulder, ft	12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking	No		0.50	No					0.50	No		0.50

HCS Signalized Intersection Results Summary

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.93									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00									
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 BD PM Peak - Int 1 - Int 3.xus												
Project Description	2043 Build Condition 2 Traffic Scenario															
Demand Information					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h					75	4	211				441	330		89	775	
Signal Information																
Cycle, s	70.0	Reference Phase	2													
Offset, s	59	Reference Point	End													
Uncoordinated	No	Simult. Gap E/W	On													
Force Mode	Fixed	Simult. Gap N/S	On		Green	41.1	17.1	0.0	0.0	0.0	0.0					
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0								
		Red	1.3	1.0	0.0	0.0	0.0	0.0								
Timer Results					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase						4				2		6				
Case Number						11.0				7.0		8.0				
Phase Duration, s						22.5				47.5		47.5				
Change Period, (Y+R _c), s						5.4				6.4		6.4				
Max Allow Headway (MAH), s						3.3				0.0		0.0				
Queue Clearance Time (g _s), s						16.5										
Green Extension Time (g _e), s						0.6				0.0		0.0				
Phase Call Probability						1.00										
Max Out Probability						0.00										
Movement Group Results					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement					7	4	14				2	12	1	6		
Adjusted Flow Rate (v), veh/h						85	227				306	229		960		
Adjusted Saturation Flow Rate (s), veh/h/ln						1189	1055				1518	1286		1485		
Queue Service Time (g _s), s						4.1	14.5				11.1	8.5		30.1		
Cycle Queue Clearance Time (g _c), s						4.1	14.5				11.1	8.5		41.1		
Green Ratio (g/C)						0.24	0.24				0.59	0.59		0.59		
Capacity (c), veh/h						290	257				892	756		930		
Volume-to-Capacity Ratio (X)						0.293	0.883				0.343	0.303		1.032		
Back of Queue (Q), ft/ln (95 th percentile)																
Back of Queue (Q), veh/ln (95 th percentile)						1.9	6.5				6.6	3.9		14.1		
Queue Storage Ratio (RQ) (95 th percentile)						0.04	0.85				0.75	0.45		0.95		
Uniform Delay (d ₁), s/veh						21.6	25.5				14.0	11.3		8.5		
Incremental Delay (d ₂), s/veh						0.2	3.9				0.9	0.8		28.8		
Initial Queue Delay (d ₃), s/veh						0.0	0.0				0.0	0.0		0.0		
Control Delay (d), s/veh						21.8	29.4				14.9	12.1		37.3		
Level of Service (LOS)						C	C				B	B		F		
Approach Delay, s/veh / LOS					27.4	C	0.0		13.7	B	37.3	D				
Intersection Delay, s/veh / LOS					28.6			C								
Multimodal Results					EB			WB			NB			SB		
Pedestrian LOS Score / LOS					1.94	B	1.71	B	1.35	A	1.64	B				
Bicycle LOS Score / LOS					1.00	A			1.86	B	2.02	B				

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	211					441	330	89	775	

Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	59	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On	Green	41.1	17.1	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
				Red	1.3	1.0	0.0	0.0	0.0	0.0					

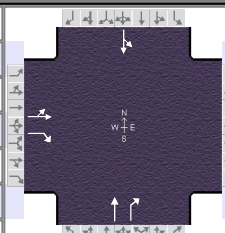
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.955	0.955					1.000	1.000		0.913	0.913	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847						0.000	0.847		0.000
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h	1128	60	1055				0	1518	1286	153	1332	0
Proportion of Vehicles Arriving on Green (P)	0.24	0.24	0.24	0.00	0.00	0.00	0.00	0.30	0.40	0.49	0.83	0.00
Incremental Delay Factor (k)		0.04	0.04					0.50	0.50		0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0				6.4		6.4
Green Ratio (g/C)		0.24				0.59		0.59
Permitted Saturation Flow Rate (s_p), veh/h/ln		0				652		1090
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						1518		0
Permitted Effective Green Time (g_p), s		0.0				0.0		41.1
Permitted Service Time (g_u), s		0.0				0.0		30.1
Permitted Queue Service Time (g_{ps}), s								30.1
Time to First Blockage (g_f), s		0.0				41.1		9.2
Queue Service Time Before Blockage (g_{ts}), s								9.2
Protected Right Saturation Flow (s_R), veh/h/ln		0				0		
Protected Right Effective Green Time (g_R), s		0.0				0.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	1.198	0.000	0.972	0.000	0.681	0.000	0.972	0.000				
Pedestrian F_s / F_{delay}	0.000	0.143	0.000	0.143	0.000	0.071	0.000	0.071				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		41.69		40.18	1175.64	5.95	1175.64	5.95				
Bicycle F_w / F_v	-3.64	0.51	-3.64		-3.64	1.37	-3.64	1.53				

HCS Signalized Intersection Results Graphical Summary

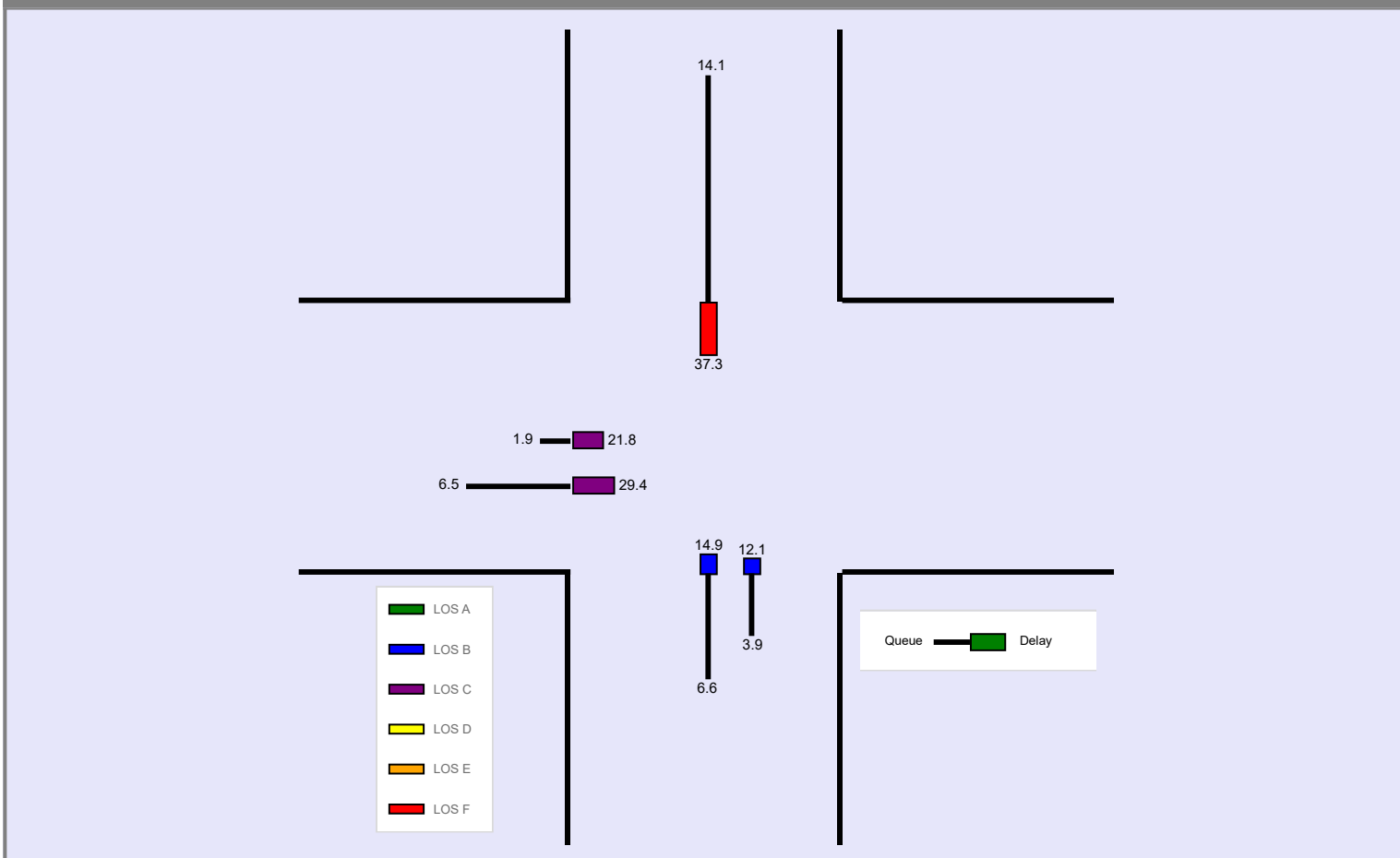
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB					
	L	T	R	L	T	R	L	T	R	L	T	R			
Approach Movement															
Demand (v), veh/h	75	4	211							441	330		89	775	

Signal Information														
Cycle, s	70.0	Reference Phase	2											
Offset, s	59	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On											
Force Mode	Fixed	Simult. Gap N/S	On											
				Green	41.1	17.1	0.0	0.0	0.0	0.0				
				Yellow	5.1	4.4	0.0	0.0	0.0	0.0				
				Red	1.3	1.0	0.0	0.0	0.0	0.0				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		1.9	6.5					6.6	3.9			14.1
Queue Storage Ratio (RQ) (95 th percentile)		0.04	0.85					0.75	0.45			0.95
Control Delay (d), s/veh		21.8	29.4					14.9	12.1			37.3
Level of Service (LOS)		C	C					B	B			F
Approach Delay, s/veh / LOS	27.4	C		0.0				13.7	B		37.3	D
Intersection Delay, s/veh / LOS	28.6						C					



--- Messages ---

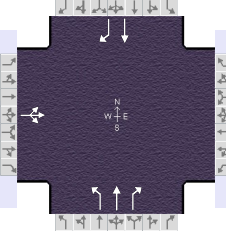
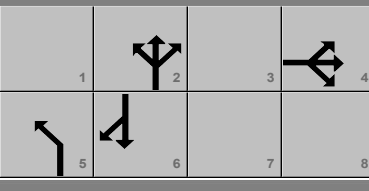
WARNING: According to input data, upstream feeding volume is equal to 65% of downstream exit volume during time period #1, for thru movement #2.

--- Comments ---

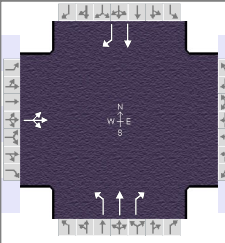
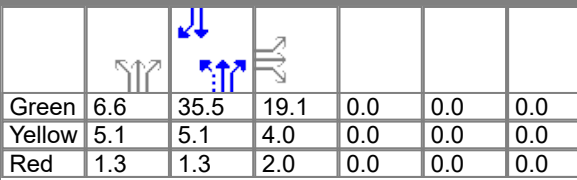
HCS Signalized Intersection Input Data

General Information					Intersection Information												
Agency	CESO				Duration, h	0.250											
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other										
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.90										
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00										
Intersection	SR-149 & Reco Drive		File Name	2043 BD AM Peak - Int 1 - Int 3.xus													
Project Description	2043 Build Condition 2 Traffic Scenario																
Demand Information					EB			WB			NB			SB			
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h					205	0	107				115	284	366			351	238
Signal Information																	
Cycle, s	80.0	Reference Phase	2														
Offset, s	10	Reference Point	End														
Uncoordinated	No	Simult. Gap E/W	On														
Force Mode					Fixed	Simult. Gap N/S	On										
Green					6.6	35.5	19.1	0.0	0.0	0.0							
Yellow					5.1	5.1	4.0	0.0	0.0	0.0							
Red					1.3	1.3	2.0	0.0	0.0	0.0							
Traffic Information					EB			WB			NB			SB			
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h					205	0	107				115	284	366			351	238
Initial Queue (Q _b), veh/h					0	0	0				0	0	0			0	0
Base Saturation Flow Rate (s ₀), veh/h					1750	1750	1750				1750	1750	1750			1750	1750
Parking (N _m), man/h					None						None					None	
Heavy Vehicles (P _{HV}), %					0						15	15	15			20	20
Ped / Bike / RTOR, /h					0	0	0	0	0		0	0	0	0	0	0	0
Buses (N _b), buses/h					0	0	0				0	0	0	0	0	0	0
Arrival Type (AT)					3	3	3				3	3	3			3	3
Upstream Filtering (I)					1.00	1.00	1.00				1.00	1.00	1.00			0.75	0.75
Lane Width (W), ft					12.0						12.0	12.0	12.0			12.0	12.0
Turn Bay Length, ft					500						175	175	175			250	200
Grade (P _g), %					0				0		0				0		
Speed Limit, mi/h					25	25	25				45	45	45			45	45
Phase Information					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s						28.0			14.0	52.0		38.0					
Yellow Change Interval (Y), s						4.0			5.1	5.1		5.1					
Red Clearance Interval (R _c), s						2.0			1.3	1.3		1.3					
Minimum Green (G _{min}), s						10			7	20		20					
Start-Up Lost Time (l _t), s					2.0	2.0			2.0	2.0		2.0					
Extension of Effective Green (e), s					2.0	2.0			2.0	2.0		2.0					
Passage (P _T), s						2.0			2.0	2.0		2.0					
Recall Mode						Off			Off	Min		Min					
Dual Entry						Yes			No	Yes		Yes					
Walk (Walk), s						0.0		0.0				0.0					
Pedestrian Clearance Time (P _C), s						0.0		0.0				0.0					
Multimodal Information					EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius					0.0	No	25.0	0.0	No	25.0				0.0	No	25.0	
Walkway / Crosswalk Width / Length, ft					9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0	
Street Width / Island / Curb, ft					0.0	0	No		0		0.0		No	0.0	0	No	
Width Outside / Bike Lane / Shoulder, ft					12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking					No	0.50		No				0.50		No	0.50		

HCS Signalized Intersection Results Summary

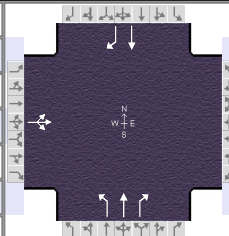
General Information						Intersection Information									
Agency	CESO					Duration, h	0.250								
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other								
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.90								
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00								
Intersection	SR-149 & Reco Drive		File Name	2043 BD AM Peak - Int 1 - Int 3.xus											
Project Description	2043 Build Condition 2 Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				205	0	107				115	284	366		351	238
Signal Information															
Cycle, s	80.0	Reference Phase	2	Green	6.6	35.5	19.1	0.0	0.0	0.0	1	2	3	4	
Offset, s	10	Reference Point	End	Yellow	5.1	5.1	4.0	0.0	0.0	0.0	5	6	7	8	
Uncoordinated	No	Simult. Gap E/W	On	Red	1.3	1.3	2.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4			5	2		6				
Case Number					12.0			1.0	3.0		7.3				
Phase Duration, s					25.1			13.0	54.9		41.9				
Change Period, (Y+R _c), s					6.0			6.4	6.4		6.4				
Max Allow Headway (MAH), s					3.4			3.0	0.0		0.0				
Queue Clearance Time (g _s), s					18.9			5.4							
Green Extension Time (g _e), s					0.3			0.1	0.0		0.0				
Phase Call Probability					1.00			0.94							
Max Out Probability					1.00			0.08							
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14				5	2	12		6	16
Adjusted Flow Rate (v), veh/h				347						128	316	407	424	287	
Adjusted Saturation Flow Rate (s), veh/h/ln				1599						1472	1545	1310	1477	1252	
Queue Service Time (g _s), s				16.9						3.4	8.1	14.2	19.2	11.8	
Cycle Queue Clearance Time (g _c), s				16.9						3.4	8.1	14.2	19.2	11.8	
Green Ratio (g/C)				0.24						0.55	0.61	0.61	0.44	0.44	
Capacity (c), veh/h				382						387	936	793	655	555	
Volume-to-Capacity Ratio (X)				0.907						0.330	0.337	0.513	0.647	0.517	
Back of Queue (Q), ft/ln (95 th percentile)															
Back of Queue (Q), veh/ln (95 th percentile)				13.1						1.6	4.2	6.5	10.8	5.4	
Queue Storage Ratio (RQ) (95 th percentile)				0.65						0.26	0.67	1.03	1.25	0.78	
Uniform Delay (d ₁), s/veh				29.6						12.1	7.8	9.0	20.6	13.0	
Incremental Delay (d ₂), s/veh				19.2						0.2	1.0	2.4	3.7	2.6	
Initial Queue Delay (d ₃), s/veh				0.0						0.0	0.0	0.0	0.0	0.0	
Control Delay (d), s/veh				48.8						12.3	8.8	11.4	24.3	15.6	
Level of Service (LOS)				D						B	A	B	C	B	
Approach Delay, s/veh / LOS				48.8	D	0.0				10.6	B	20.8	C		
Intersection Delay, s/veh / LOS				21.3						C					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.14	B	1.95	B	1.35	A	1.67	B				
Bicycle LOS Score / LOS				1.06	A			1.89	B	1.57	B				

HCS Signalized Intersection Intermediate Values

General Information					Intersection Information												
Agency	CESO				Duration, h	0.250											
Analyst	TMC		Analysis Date	11/18/2022		Area Type	Other										
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.90										
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00										
Intersection	SR-149 & Reco Drive		File Name	2043 BD AM Peak - Int 1 - Int 3.xus													
Project Description	2043 Build Condition 2 Traffic Scenario																
Demand Information					EB			WB			NB			SB			
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h					205	0	107				115	284	366			351	238
Signal Information										1		2		3		4	
Cycle, s	80.0	Reference Phase	2														
Offset, s	10	Reference Point	End														
Uncoordinated	No	Simult. Gap E/W	On														
Force Mode	Fixed	Simult. Gap N/S	On														
Green					6.6	35.5	19.1	0.0	0.0	0.0							
Yellow					5.1	5.1	4.0	0.0	0.0	0.0							
Red					1.3	1.3	2.0	0.0	0.0	0.0							
Saturation Flow / Delay					L	T	R	L	T	R	L	T	R	L	T	R	
Lane Width Adjustment Factor (f _w)																	
Heavy Vehicles and Grade Factor (f _{HVg})																	
Parking Activity Adjustment Factor (f _p)					1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	
Bus Blockage Adjustment Factor (f _{bb})					1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	
Area Type Adjustment Factor (f _a)																	
Lane Utilization Adjustment Factor (f _{LU})					1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Left-Turn Adjustment Factor (f _{LT})					0.914	0.914					0.952	0.000		1.000	1.000		
Right-Turn Adjustment Factor (f _{RT})						0.000	0.000					0.000	0.847		0.000	0.847	
Left-Turn Pedestrian Adjustment Factor (f _{LPB})					1.000						1.000			1.000			
Right-Turn Ped-Bike Adjustment Factor (f _{RPB})							1.000						1.000			1.000	
Work Zone Adjustment Factor (f _{wz})																	
DDI Factor (f _{DDI})																	
Movement Saturation Flow Rate (s), veh/h					1050	0	548				1472	1545	1310	0	1477	1252	
Proportion of Vehicles Arriving on Green (P)					0.24	0.00	0.24	0.00	0.00	0.00	0.08	0.61	0.61	0.00	0.35	0.54	
Incremental Delay Factor (k)						0.31					0.04	0.50	0.50		0.50	0.50	
Signal Timing / Movement Groups					EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R					
Lost Time (t _L)						4.0			6.4	6.4		6.4					
Green Ratio (g/C)						0.24			0.55	0.61		0.44					
Permitted Saturation Flow Rate (s _p), veh/h/ln						0			864	0		1081					
Shared Saturation Flow Rate (s _{sh}), veh/h/ln												0					
Permitted Effective Green Time (g _p), s						0.0			37.5	0.0		0.0					
Permitted Service Time (g _u), s						0.0			16.3	0.0		0.0					
Permitted Queue Service Time (g _{ps}), s									3.7								
Time to First Blockage (g _t), s						0.0			0.0	0.0		35.5					
Queue Service Time Before Blockage (g _{ts}), s																	
Protected Right Saturation Flow (s _R), veh/h/ln										0		0					
Protected Right Effective Green Time (g _R), s										0.0		0.0					
Multimodal					EB			WB			NB			SB			
Pedestrian F _w / F _v					1.389	0.000	1.198	0.000	0.681	0.000	0.972	0.000					
Pedestrian F _s / F _{delay}					0.000	0.148	0.000	0.148	0.000	0.073	0.000	0.101					
Pedestrian M _{corner} / M _{cw}					0.00		0.00		0.00		0.00						
Bicycle c _b / d _b						47.31		45.16	1211.84	6.21	887.07	12.39					
Bicycle F _w / F _v					-3.64	0.57	-3.64		-3.64	1.40	-3.64	1.08					

HCS Signalized Intersection Results Graphical Summary

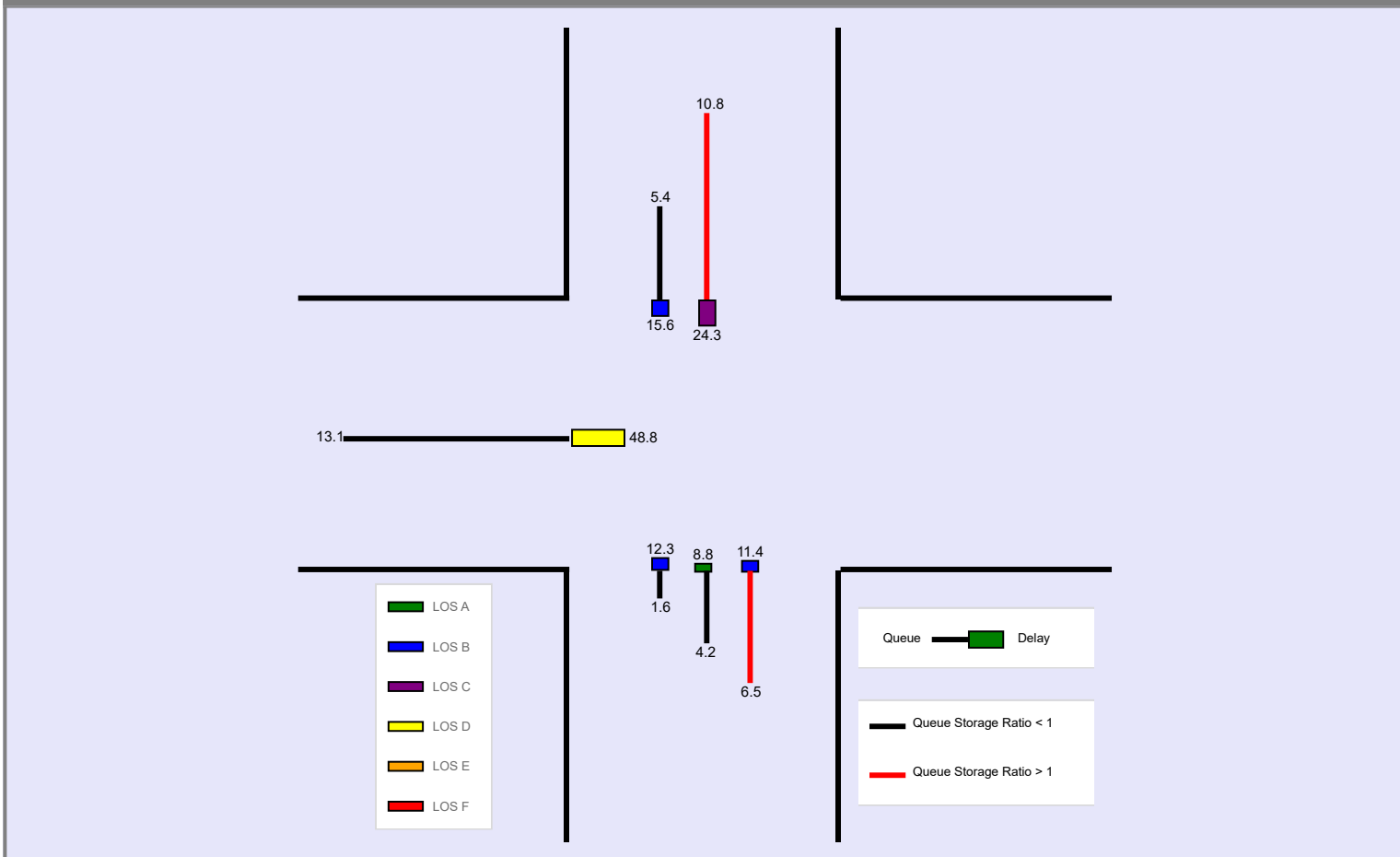
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & Reco Drive	File Name	2043 BD AM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	205	0	107				115	284	366		351	238

Signal Information				Signal Phases											
Cycle, s	80.0	Reference Phase	2												
Offset, s	10	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
		Green	6.6	35.5	19.1	0.0	0.0	0.0							
		Yellow	5.1	5.1	4.0	0.0	0.0	0.0							
		Red	1.3	1.3	2.0	0.0	0.0	0.0							

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		13.1					1.6	4.2	6.5		10.8	5.4
Queue Storage Ratio (RQ) (95 th percentile)		0.65					0.26	0.67	1.03		1.25	0.78
Control Delay (d), s/veh		48.8					12.3	8.8	11.4		24.3	15.6
Level of Service (LOS)		D					B	A	B		C	B
Approach Delay, s/veh / LOS	48.8		D	0.0			10.6		B	20.8		C
Intersection Delay, s/veh / LOS	21.3						C					

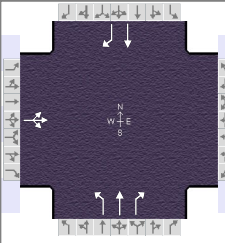


--- Messages ---

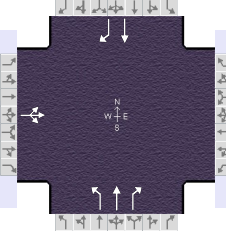
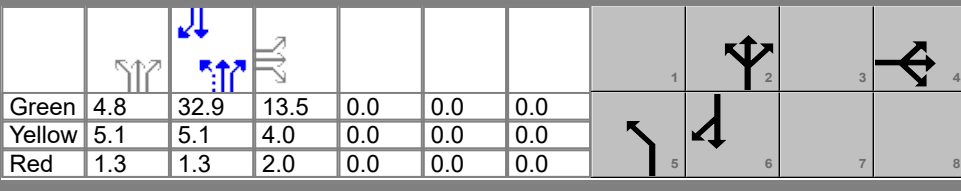
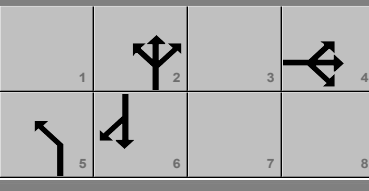
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

HCS Signalized Intersection Input Data

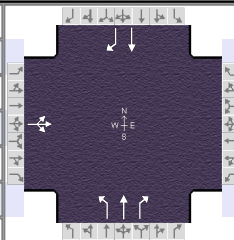
General Information						Intersection Information										
Agency		CESO				Duration, h		0.250								
Analyst		TMC		Analysis Date		11/18/2022		Area Type		Other						
Jurisdiction		ODOT		Time Period		AM Peak Hour		PHF		0.93						
Urban Street		SR-149		Analysis Year		2043		Analysis Period		1 > 7:00						
Intersection		SR-149 & Reco Drive		File Name		2043 BD PM Peak - Int 1 - Int 3.xus										
Project Description		2043 Build Condition 2 Traffic Scenario														
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				190	0	62				56	308	273		797	189	
Signal Information																
Cycle, s		70.0		Reference Phase		2										
Offset, s		43		Reference Point		End										
Uncoordinated		No		Simult. Gap E/W		On										
Force Mode		Fixed		Simult. Gap N/S		On										
				Green	4.8	32.9	13.5	0.0	0.0	0.0						
				Yellow	5.1	5.1	4.0	0.0	0.0	0.0						
				Red	1.3	1.3	2.0	0.0	0.0	0.0						
Traffic Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				190	0	62				56	308	273		797	189	
Initial Queue (Q _b), veh/h				0	0	0				0	0	0		0	0	
Base Saturation Flow Rate (s ₀), veh/h				1750	1750	1750				1750	1750	1750		1750	1750	
Parking (N _m), man/h				None						None			None			
Heavy Vehicles (P _{HV}), %				1						21	21	21	18 18			
Ped / Bike / RTOR, /h				0	0	0	0	0		0	0	0	0	0	0	
Buses (N _b), buses/h				0	0	0				0	0	0	0	0	0	
Arrival Type (AT)				3	3	3				3	3	3		3	3	
Upstream Filtering (I)				1.00	1.00	1.00				1.00	1.00	1.00		0.22	0.22	
Lane Width (W), ft				12.0						12.0	12.0	12.0	12.0 12.0			
Turn Bay Length, ft				500						175	175	175	250 200			
Grade (P _g), %				0			0			0			0			
Speed Limit, mi/h				25	25	25				45	45	45		45	45	
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s					20.0			14.0	50.0		36.0					
Yellow Change Interval (Y), s					4.0			5.1	5.1		5.1					
Red Clearance Interval (R _c), s					2.0			1.3	1.3		1.3					
Minimum Green (G _{min}), s					10			7	20		20					
Start-Up Lost Time (l _t), s				2.0	2.0			2.0	2.0		2.0					
Extension of Effective Green (e), s				2.0	2.0			2.0	2.0		2.0					
Passage (P _T), s					2.0			2.0	2.0		2.0					
Recall Mode					Off			Off	Min		Min					
Dual Entry					Yes			No	Yes		Yes					
Walk (Walk), s					0.0		0.0				0.0					
Pedestrian Clearance Time (P _C), s					0.0		0.0				0.0					
Multimodal Information				EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0				0.0	No	25.0	
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0	
Street Width / Island / Curb, ft				0.0	0	No		0		0.0		No	0.0	0	No	
Width Outside / Bike Lane / Shoulder, ft				12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking				No	0.50		No				0.50		No	0.50		

HCS Signalized Intersection Results Summary

General Information				Intersection Information											
Agency	CESO			Duration, h	0.250										
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other										
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93										
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00										
Intersection	SR-149 & Reco Drive	File Name	2043 BD PM Peak - Int 1 - Int 3.xus												
Project Description	2043 Build Condition 2 Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				190	0	62				56	308	273		797	189
Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	43	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
Green	4.8	32.9	13.5	0.0	0.0	0.0									
Yellow	5.1	5.1	4.0	0.0	0.0	0.0									
Red	1.3	1.3	2.0	0.0	0.0	0.0									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase					4			5	2		6				
Case Number					12.0			1.0	3.0		7.3				
Phase Duration, s					19.5			11.2	50.5		39.3				
Change Period, (Y+R _c), s					6.0			6.4	6.4		6.4				
Max Allow Headway (MAH), s					3.3			3.0	0.0		0.0				
Queue Clearance Time (g _s), s					13.5			3.4							
Green Extension Time (g _e), s					0.0			0.0	0.0		0.0				
Phase Call Probability					0.99			0.69							
Max Out Probability					1.00			0.06							
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14				5	2	12		6	16
Adjusted Flow Rate (v), veh/h				271						60	331	294	834 198		
Adjusted Saturation Flow Rate (s), veh/h/ln				1605						1394	1463	1240	1504 1275		
Queue Service Time (g _s), s				11.5						1.4	7.6	8.0	32.9 8.7		
Cycle Queue Clearance Time (g _c), s				11.5						1.4	7.6	8.0	32.9 8.7		
Green Ratio (g/C)				0.19						0.57	0.63	0.63	0.47 0.47		
Capacity (c), veh/h				308						199	923	782	707 599		
Volume-to-Capacity Ratio (X)				0.878						0.302	0.359	0.375	1.179 0.330		
Back of Queue (Q), ft/ln (95 th percentile)															
Back of Queue (Q), veh/ln (95 th percentile)				10.2						0.7	3.3	3.0	33.9 3.6		
Queue Storage Ratio (RQ) (95 th percentile)				0.51						0.12	0.55	0.51	3.87 0.51		
Uniform Delay (d ₁), s/veh				27.5						15.5	6.2	6.3	19.1 16.7		
Incremental Delay (d ₂), s/veh				21.6						0.3	1.1	1.4	84.2 0.3		
Initial Queue Delay (d ₃), s/veh				0.0						0.0	0.0	0.0	0.0 0.0		
Control Delay (d), s/veh				49.1						15.9	7.3	7.6	103.3 17.1		
Level of Service (LOS)				D						B	A	A	F B		
Approach Delay, s/veh / LOS				49.1	D	0.0			8.2	A	86.8	F			
Intersection Delay, s/veh / LOS				54.6						D					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				2.13	B	1.94	B	1.34	A	1.66	B				
Bicycle LOS Score / LOS				0.93	A			1.62	B	2.24	B				

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & Reco Drive	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	190	0	62				56	308	273		797	189

Signal Information													
Cycle, s	70.0	Reference Phase	2										
Offset, s	43	Reference Point	End										
Uncoordinated	No	Simult. Gap E/W	On	Green	4.8	32.9	13.5	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	5.1	4.0	0.0	0.0	0.0			
				Red	1.3	1.3	2.0	0.0	0.0	0.0			

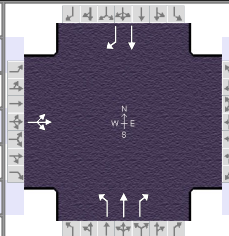
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.924	0.924					0.952	0.000		1.000	1.000	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.000					0.000	0.847		0.000	0.847
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h	1210	0	395				1394	1463	1240	0	1504	1275
Proportion of Vehicles Arriving on Green (P)	0.19	0.00	0.19	0.00	0.00	0.00	0.07	0.63	0.63	0.00	0.45	0.27
Incremental Delay Factor (k)		0.36					0.04	0.50	0.50		0.50	0.50

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0			6.4	6.4		6.4
Green Ratio (g/C)		0.19			0.57	0.63		0.47
Permitted Saturation Flow Rate (s_p), veh/h/ln		0			559	0		1066
Shared Saturation Flow Rate (s_{sh}), veh/h/ln								0
Permitted Effective Green Time (g_p), s		0.0			34.9	0.0		0.0
Permitted Service Time (g_u), s		0.0			0.0	0.0		0.0
Permitted Queue Service Time (g_{ps}), s					0.0			
Time to First Blockage (g_t), s		0.0			0.0	0.0		32.9
Queue Service Time Before Blockage (g_{ts}), s								
Protected Right Saturation Flow (s_R), veh/h/ln						0		0
Protected Right Effective Green Time (g_R), s						0.0		0.0

Multimodal	EB		WB		NB		SB	
Pedestrian F_w / F_v	1.389	0.000	1.198	0.000	0.681	0.000	0.972	0.000
Pedestrian F_s / F_{delay}	0.000	0.143	0.000	0.143	0.000	0.063	0.000	0.092
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00	
Bicycle c_b / d_b	-200.00	42.35		40.18	1261.29	4.77	940.45	9.82
Bicycle F_w / F_v	-3.64	0.45	-3.64		-3.64	1.13	-3.64	1.75

HCS Signalized Intersection Results Graphical Summary

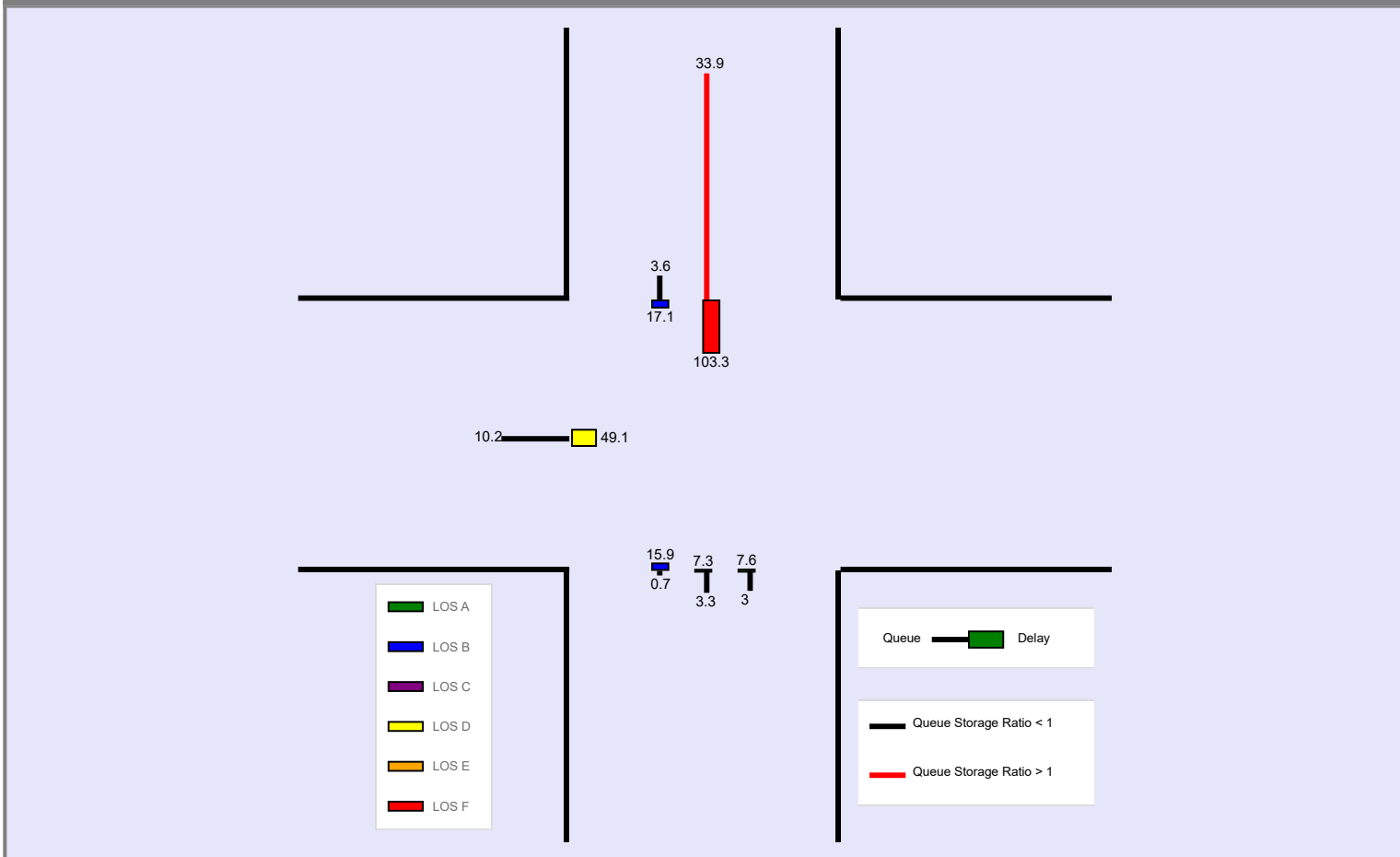
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	TMC	Analysis Date	11/18/2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & Reco Drive	File Name	2043 BD PM Peak - Int 1 - Int 3.xus				
Project Description	2043 Build Condition 2 Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	190	0	62				56	308	273		797	189

Signal Information				Signal Timing (s)										
Cycle, s	70.0	Reference Phase	2											
Offset, s	43	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	4.8	32.9	13.5	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	5.1	4.0	0.0	0.0	0.0				
				Red	1.3	1.3	2.0	0.0	0.0	0.0				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		10.2					0.7	3.3	3.0		33.9	3.6
Queue Storage Ratio (RQ) (95 th percentile)		0.51					0.12	0.55	0.51		3.87	0.51
Control Delay (d), s/veh		49.1					15.9	7.3	7.6		103.3	17.1
Level of Service (LOS)		D					B	A	A		F	B
Approach Delay, s/veh / LOS	49.1		D	0.0			8.2		A	86.8		F
Intersection Delay, s/veh / LOS	54.6						D					



--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

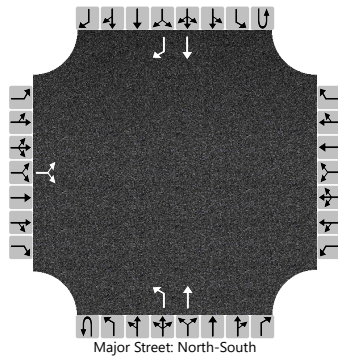
WARNING: If demand exceeds capacity, a multiple-period analysis should be conducted.

--- Comments ---

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Love's Site Driveway #2		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/20/2022			East/West Street	Love's Site Driveway #2		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	AM Peak Hour			Peak Hour Factor	0.92		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 2						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	1
Configuration			LR							L	T				T	R
Volume (veh/h)		51		17						17	714				408	50
Percent Heavy Vehicles (%)		100		100						11						
Proportion Time Blocked																
Percent Grade (%)		0														
Right Turn Channelized															No	
Median Type Storage		Undivided														

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.40		7.20						4.21						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		4.40		4.20						2.30						

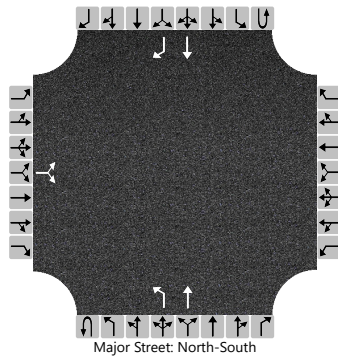
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			74							18						
Capacity, c (veh/h)			146							1021						
v/c Ratio			0.51							0.02						
95% Queue Length, Q ₉₅ (veh)			2.4							0.1						
Control Delay (s/veh)			52.8							8.6						
Level of Service (LOS)			F							A						
Approach Delay (s/veh)		52.8								0.2						
Approach LOS		F								A						

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Love's Site Driveway #2		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/20/2022			East/West Street	Love's Site Driveway #2		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	PM Peak Hour			Peak Hour Factor	0.92		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	Build Condition 2						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	0	0	0	1	1	0	0	0	1	1
Configuration			LR							L	T				T	R
Volume (veh/h)		59		10						12	578				794	65
Percent Heavy Vehicles (%)		100		100						15						
Proportion Time Blocked																
Percent Grade (%)	0															
Right Turn Channelized													No			
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.40		7.20						4.25						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		4.40		4.20						2.34						

Delay, Queue Length, and Level of Service

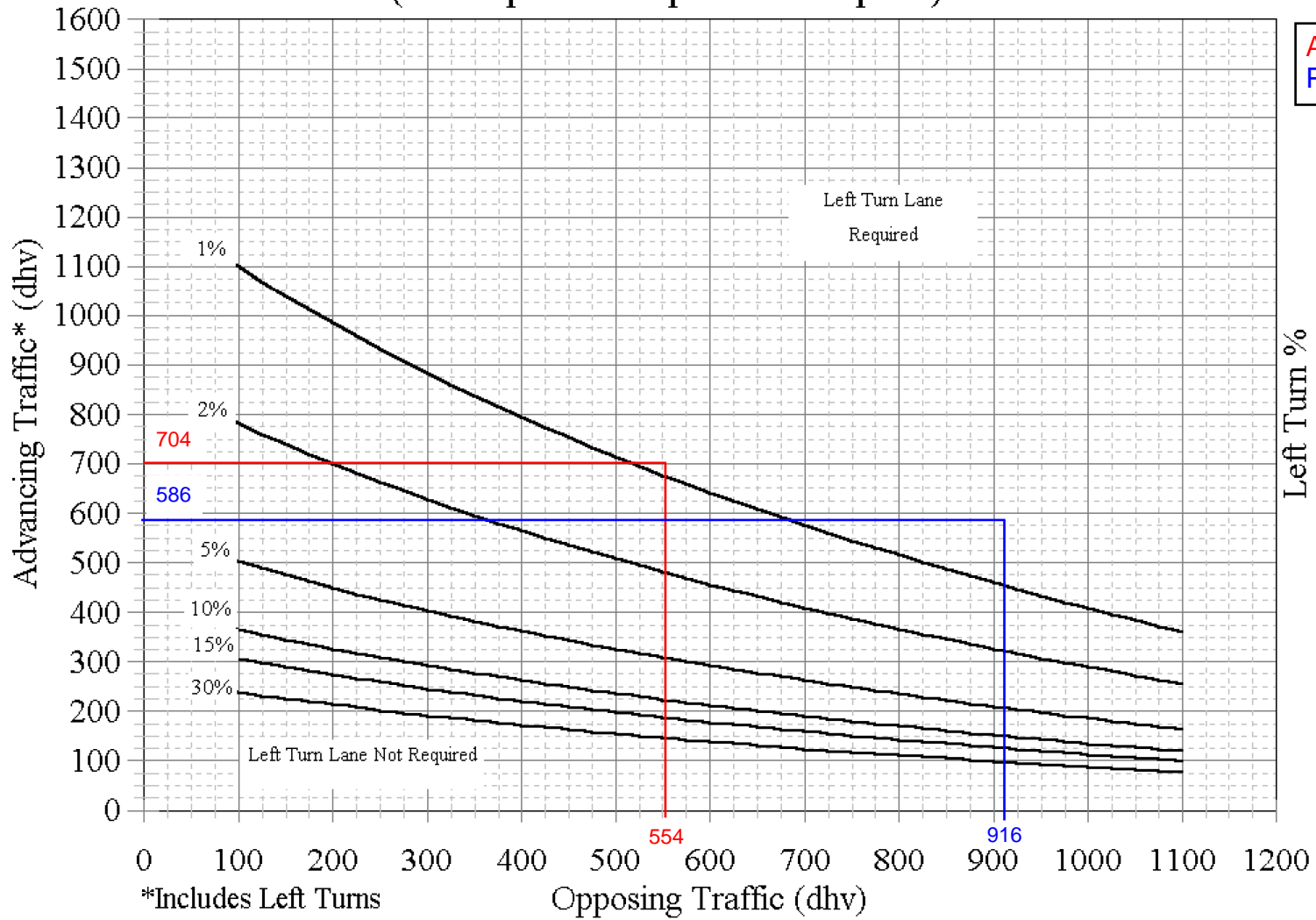
Flow Rate, v (veh/h)			75							13						
Capacity, c (veh/h)			86							683						
v/c Ratio			0.87							0.02						
95% Queue Length, Q ₉₅ (veh)			4.6							0.1						
Control Delay (s/veh)			147.7							10.4						
Level of Service (LOS)			F							B						
Approach Delay (s/veh)	147.7								0.2							
Approach LOS	F								A							

**APPENDIX G
ODOT TURN LANE RESOURCES AND
STORAGE LENGTH CALCULATION SHEETS**

SR-149 and Reco Drive
2023 Build Traffic Scenario - NBL

2-Lane Highway Left Turn Lane Warrant (>40 mph or 70 kph Posted Speed)

October 2004



AM
PM

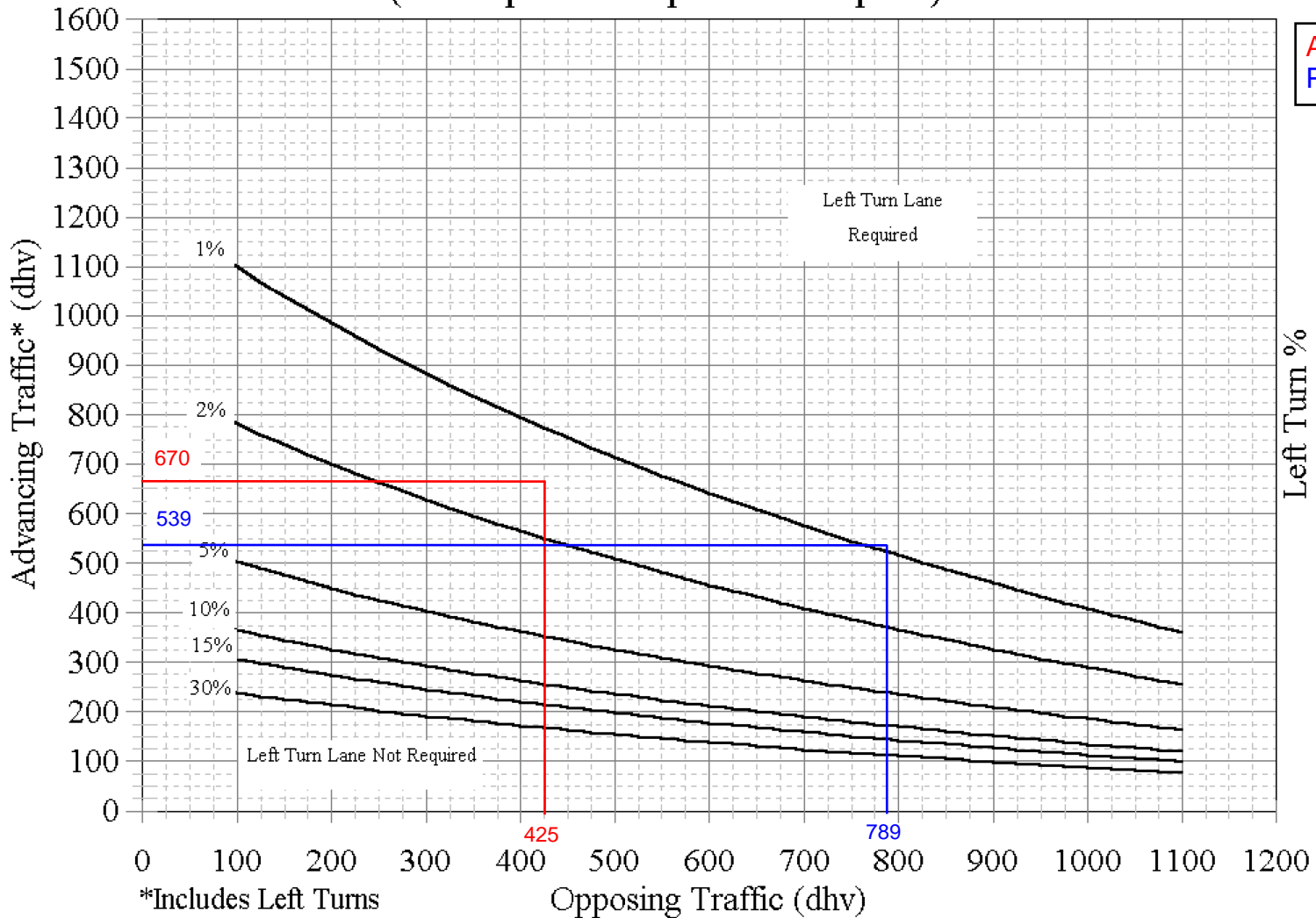
2-LANE LEFT TURN LANE
WARRANT (HIGH SPEED)

401-5b
REFERENCE SECTION
401.6.1

SR-149 and Love's Site Driveway #2
 2023 Build Traffic Scenario - NBL

2-Lane Highway Left Turn Lane Warrant
 (>40 mph or 70 kph Posted Speed)

October 2004



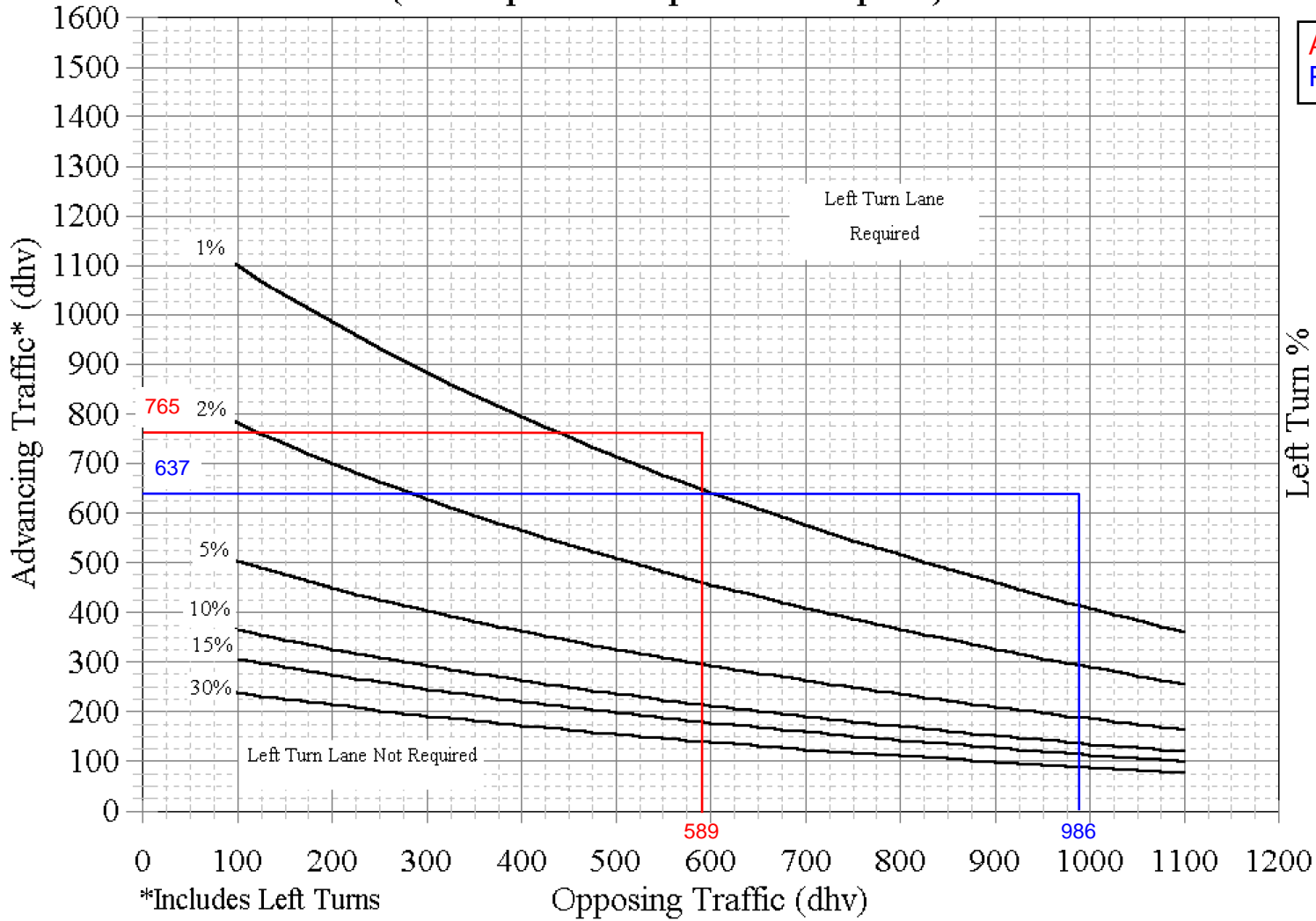
AM
PM

2-LANE LEFT TURN LANE WARRANT (HIGH SPEED)
401-5b
 REFERENCE SECTION
 401.6.1

SR-149 and Reco Drive
 2043 Design Year Traffic Scenario - NBL

2-Lane Highway Left Turn Lane Warrant (>40 mph or 70 kph Posted Speed)

October 2004



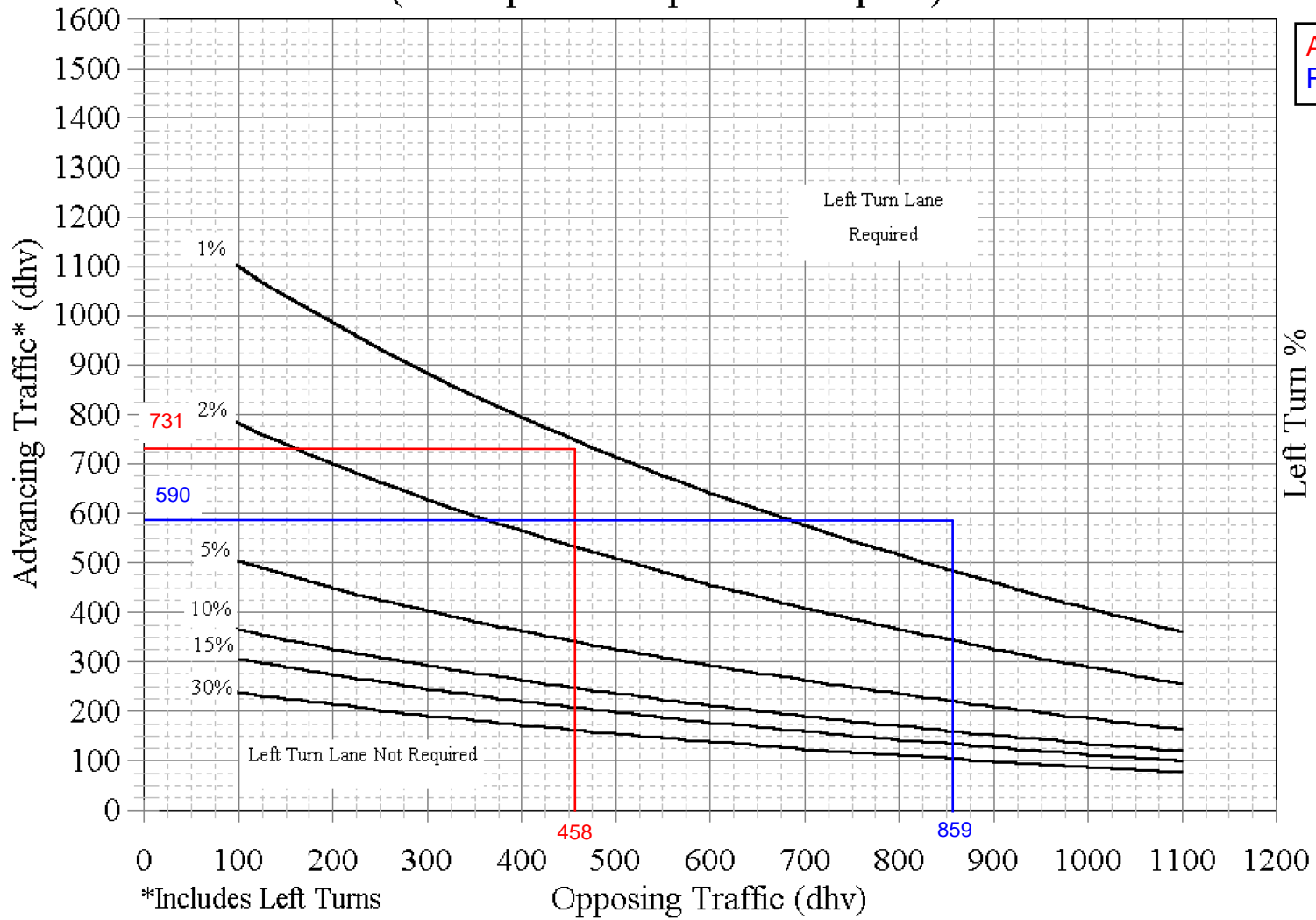
2-LANE LEFT TURN LANE
 WARRANT (HIGH SPEED)

401-5b
 REFERENCE SECTION
 401.6.1

SR-149 and Love's Site Driveway #2
 2043 Design Year Traffic Scenario - NBL

2-Lane Highway Left Turn Lane Warrant (>40 mph or 70 kph Posted Speed)

October 2004



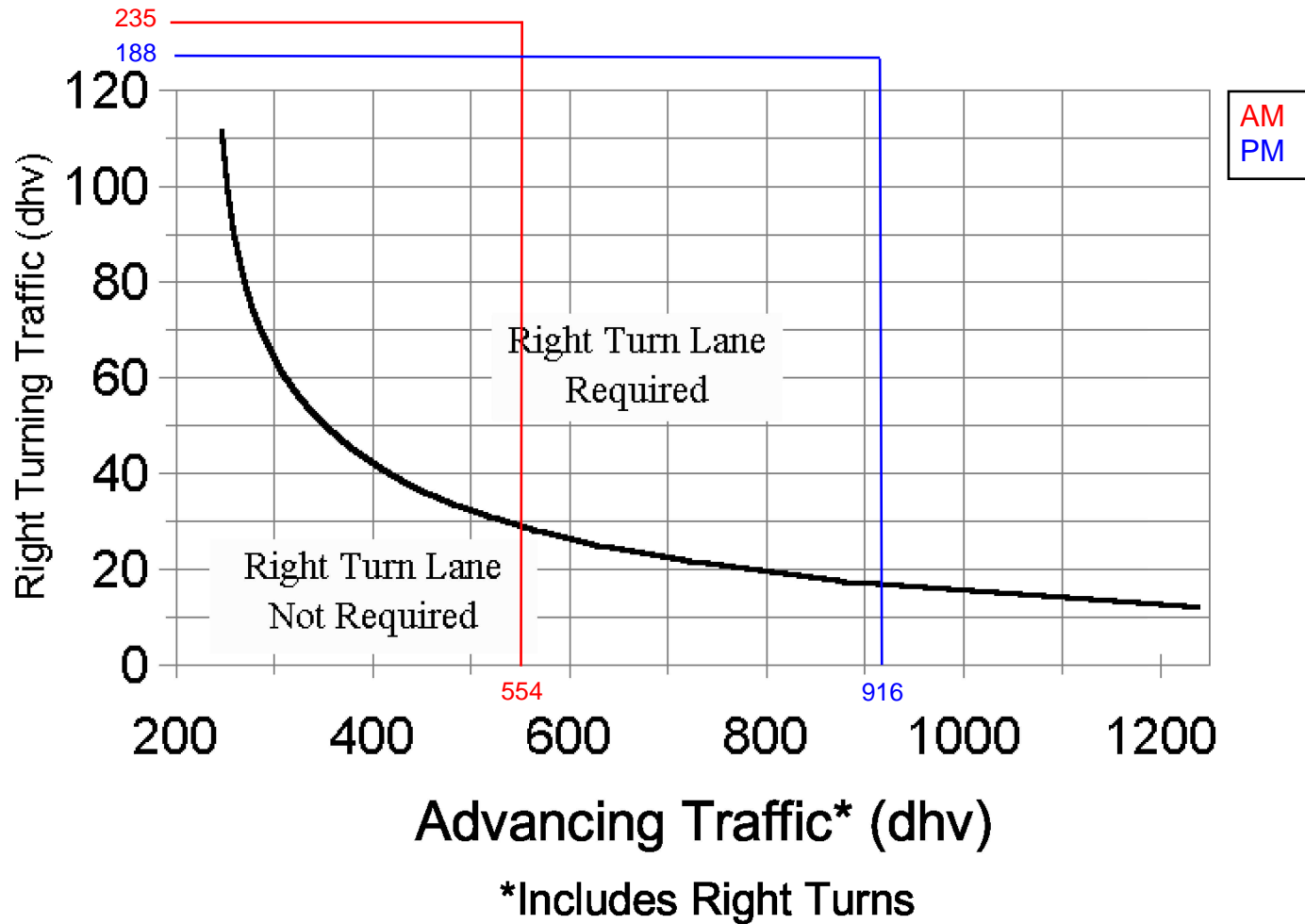
AM
 PM

**2-LANE LEFT TURN LANE
 WARRANT (HIGH SPEED)**
401-5b
 REFERENCE SECTION
 401.6.1

SR-149 and Reco Drive
2023 Build Traffic Scenario - SBR

2-Lane Highway Right Turn Lane Warrant

> 40 mph or 70 kph Posted Speed



October 2004

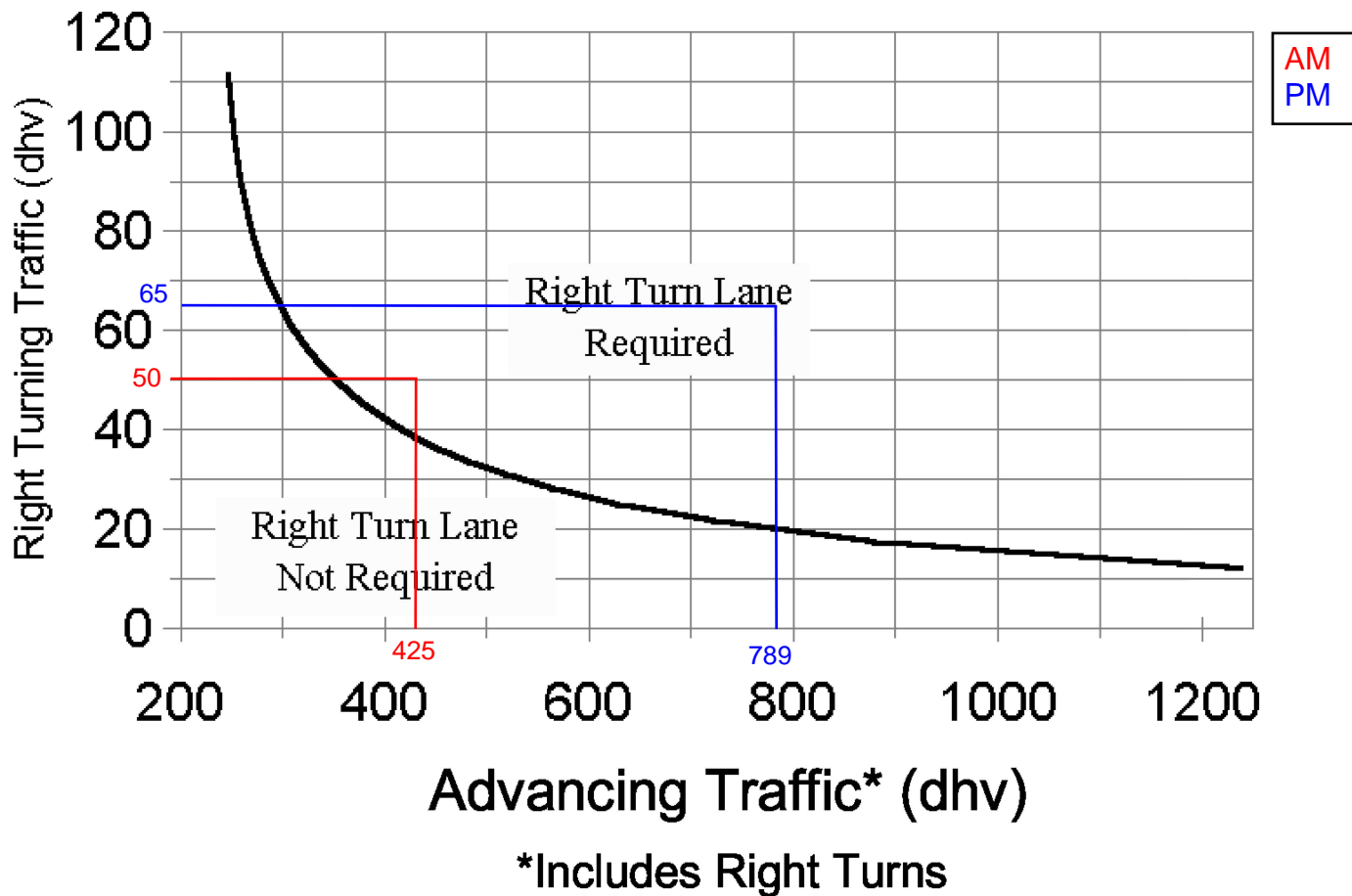
2-LANE RIGHT TURN LANE
WARRANT (HIGH SPEED)

401-6b
REFERENCE SECTION
401.6.3

SR-149 and Love's Site Driveway #2
2023 Build Traffic Scenario - SBR

2-Lane Highway Right Turn Lane Warrant

> 40 mph or 70 kph Posted Speed



October 2004

2-LANE RIGHT TURN LANE
WARRANT (HIGH SPEED)

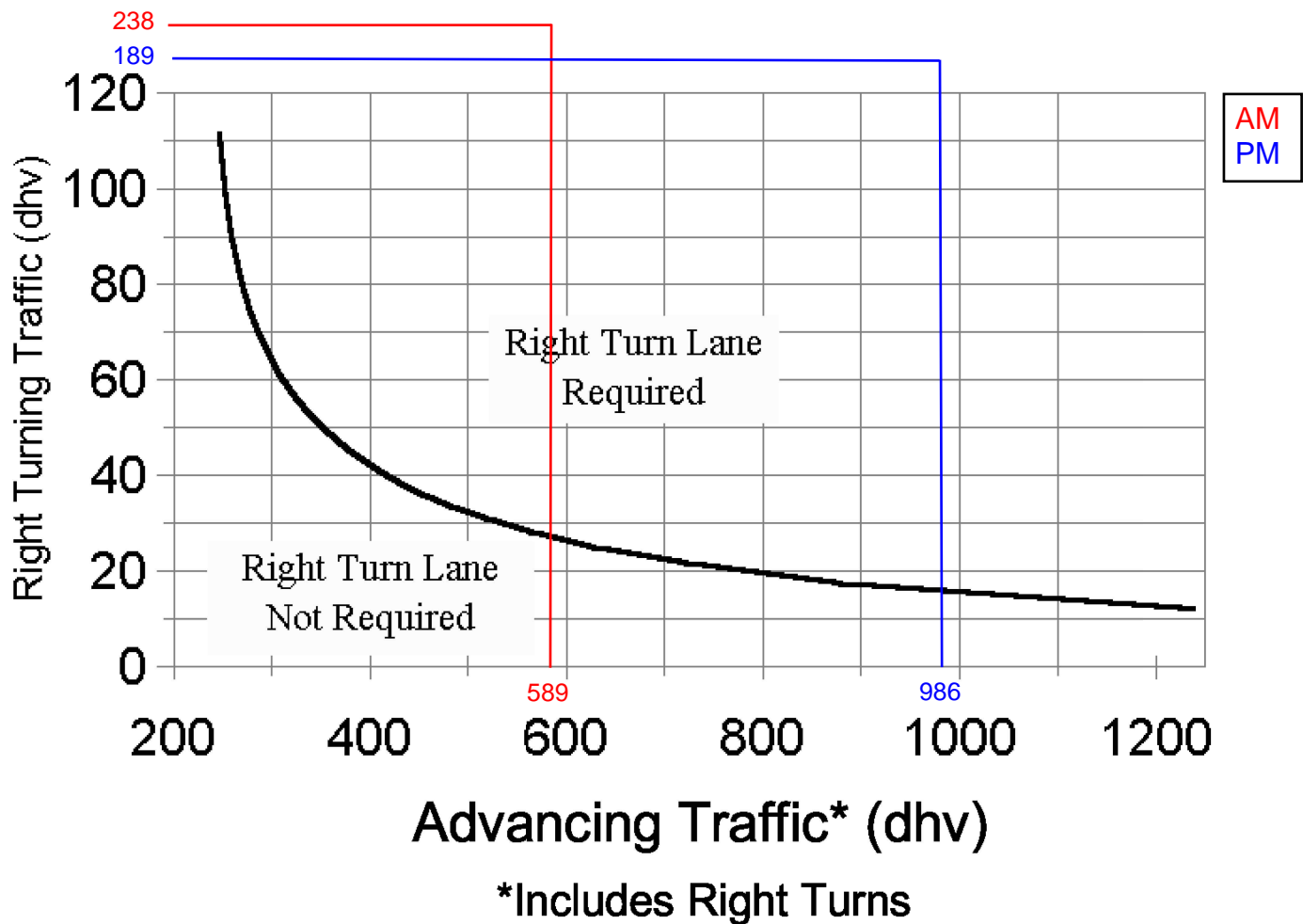
REFERENCE SECTION
401.6.3

401-6b

SR-149 and Reco Drive
2043 Design Year Traffic Scenario - SBR

2-Lane Highway Right Turn Lane Warrant

> 40 mph or 70 kph Posted Speed



October 2004

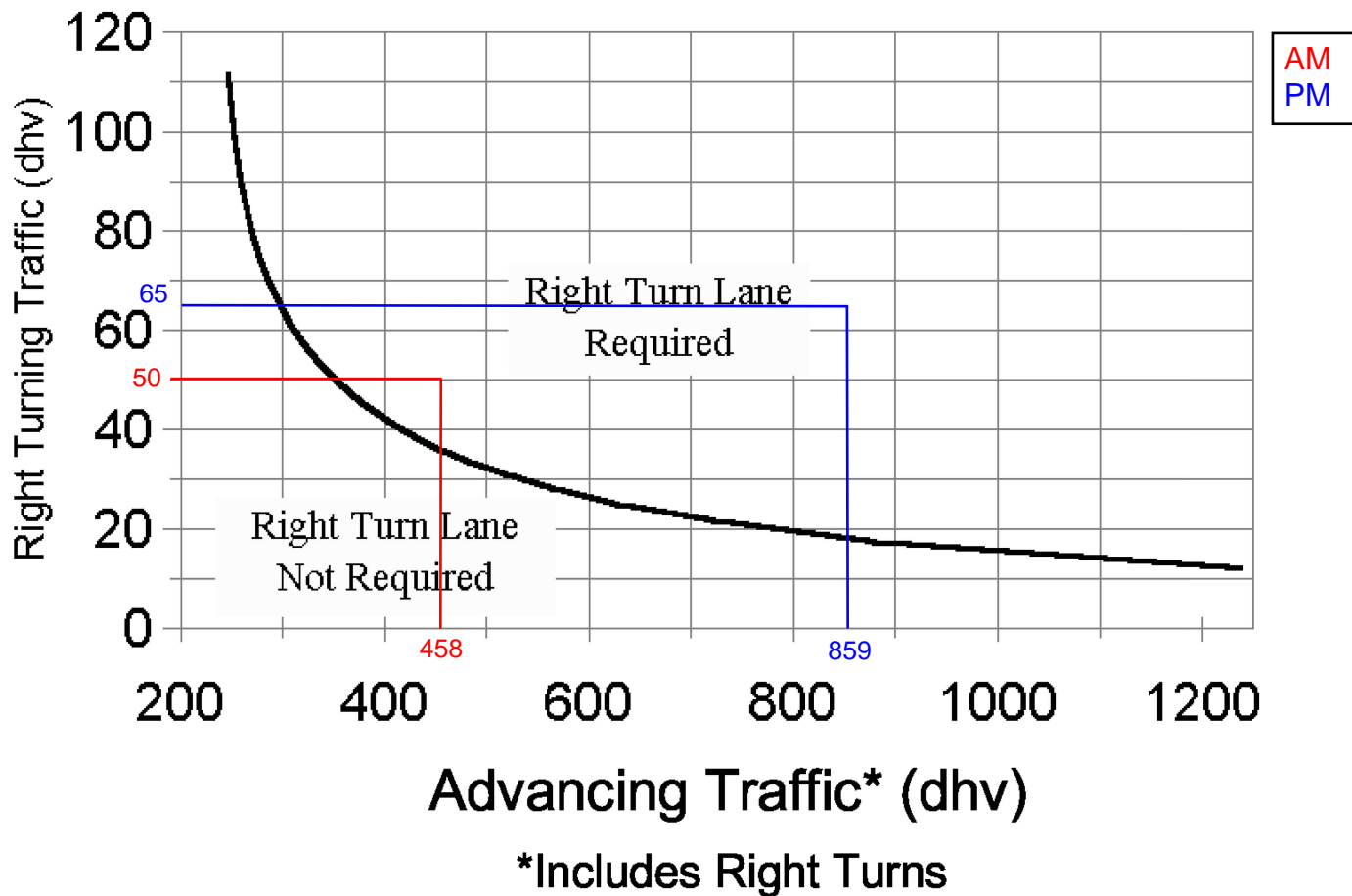
2-LANE RIGHT TURN LANE
WARRANT (HIGH SPEED)

401-6b
REFERENCE SECTION
401.6.3

SR-149 and Love's Site Driveway #2
2043 Design Year Traffic Scenario - SBR

2-Lane Highway Right Turn Lane Warrant

> 40 mph or 70 kph Posted Speed



October 2004

2-LANE RIGHT TURN LANE
WARRANT (HIGH SPEED)

REFERENCE SECTION
401.6.3

401-6b

2043 Design Year Traffic Scenario - AM Peak Hour																					
Location	Direction	No. of Lanes	Design Speed	Type of Traffic Control	Condition	Cycle Length (Sec.)	Cycles Per Hour	Volume	Avg. Veh./Cycle/Lane	Rounded Up Avg. Veh./Cycle	Fig. 401-10 Storage Length (ft)	Fig. 401-9 Condition			Backup Length (ft)	Required Turn Lane Length* (ft)	Ex. Storage Length (ft)	Prop. Storage Length* (ft)	Turn Lane Length (Ex/Prop < Required)?	HCS 95th Perc. Queue	
												A*	B*	C*							
SR-149 & I-70 WB On/Off Ramps	NBL	1	50 mph	Signalized	B or C	80	45	160	3.56	4.0	175		225	145	320		320	N/A	375	Yes	188.0
	NBT	1	50 mph	Signalized		80	45	330	7.33	8.0	325					325					188.0
SR-149 & I-70 EB On/Off Ramps	SBL	1	50 mph	Signalized	B or C	80	45	127	2.82	3.0	150		225	145	295		295	N/A	425	Yes	468.0
	SBT	1	50 mph	Signalized		80	45	441	9.80	10.0	375					375					468.0
SR-149 & Reco Drive	NBL	1	50 mph	Signalized	B or C	80	45	115	2.56	3.0	150		225	145	295		295	N/A	575	Yes	45.5
	NBT	1	50 mph	Signalized	B or C	80	45	650	14.44	15.0	525					525					117.3
	SBT	1	50 mph	Signalized	B or C	80	45	351	7.80	8.0	325					325					312.5
	SBR	1	50 mph	Signalized	B or C	80	45	238	5.29	6.0	250		225	145	395		395	N/A	395	Yes	156.0
SR-149 & Love's Site Driveway #2	NBL	1	50 mph	Unsignalized	B	N/A	60	17	0.28	1.0	50					225	N/A	225	No	2.5	
	SBR	1	50 mph	Unsignalized	B or C	N/A	60	50	0.83	1.0	50		225	145	195		225	N/A	225	No	0.0

2043 Design Year Traffic Scenario - PM Peak Hour																					
Location	Direction	No. of Lanes	Design Speed	Type of Traffic Control	Condition	Cycle Length (Sec.)	Cycles Per Hour	Volume	Avg. Veh./Cycle/Lane	Rounded Up Avg. Veh./Cycle	Fig. 401-10 Storage Length (ft)	Fig. 401-9 Condition			Backup Length (ft)	Required Turn Lane Length* (ft)	Ex. Storage Length (ft)	Prop. Storage Length* (ft)	Turn Lane Length (Ex/Prop < Required)?	HCS 95th Perc. Queue	
												A*	B*	C*							
SR-149 & I-70 WB On/Off Ramps	NBL	1	50 mph	Signalized	B or C	70	51	157	3.08	4.0	175		225	145	320		320	N/A	375	Yes	356.0
	NBT	1	50 mph	Signalized		70	51	359	7.04	8.0	325					325					356.0
SR-149 & I-70 EB On/Off Ramps	SBL	1	50 mph	Signalized	B or C	70	51	89	1.75	2.0	100		225	145	245		245	N/A	600	Yes	380.0
	SBT	1	50 mph	Signalized		70	51	775	15.20	16.0	550					550					380.0
SR-149 & Reco Drive	NBL	1	50 mph	Signalized	B or C	70	51	56	1.10	2.0	100		225	145	245		245	N/A	500	Yes	21.0
	NBT	1	50 mph	Signalized	B or C	70	51	581	11.39	12.0	450					450					96.3
	SBT	1	50 mph	Signalized	B or C	70	51	797	15.63	16.0	550					550					967.5
	SBR	1	50 mph	Signalized	B or C	70	51	189	3.71	4.0	175		225	145	320		320	N/A	600	Yes	102.0
SR-149 & Love's Site Driveway #2	NBL	1	50 mph	Unsignalized	B	N/A	60	12	0.20	1.0	50					225	N/A	225	No	2.5	
	SBR	1	50 mph	Unsignalized	B	N/A	60	65	1.08	2.0	100		225				225	N/A	225	No	0.0

* - Includes 50' taper

APPENDIX H
HCS 95TH PERCENTILE QUEUE LENGTH SHEETS

**NOTE: Refer to Appendices E & F
for 95th percentile queues included
on the HCS output sheets**

APPENDIX I
SIGNAL WARRANT STUDY CALCULATIONS

SR 149 & RECO DRIVE - EXISTING TRAFFIC COUNTS (YEAR 2022)

TIME BEGIN	SR 149 (NORTHBOUND)			SR 149 (SOUTHBOUND)			RECO DRIVE (EASTBOUND)			N/A (WESTBOUND)		
	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT
6AM	3	314	0	0	152	8	11	0	5	0	0	0
7AM	9	510	0	0	217	29	5	0	7	0	0	0
8AM	10	392	0	0	247	8	14	0	10	0	0	0
11AM	2	241	0	0	258	9	15	0	13	0	0	0
12PM	4	318	0	0	293	6	14	0	10	0	0	0
1PM	1	288	0	0	289	12	11	0	4	0	0	0
2PM	5	313	0	0	366	5	12	0	4	0	0	0
3PM	2	321	0	0	457	4	20	0	11	0	0	0
4PM	3	360	0	0	411	13	26	0	12	0	0	0
5PM	4	351	0	0	448	11	18	0	5	0	0	0

SR 149 & RECO DRIVE - DHV TRAFFIC VOLUMES (YEAR 2022)

TIME BEGIN	SR 149 (NORTHBOUND)			SR 149 (SOUTHBOUND)			RECO DRIVE (EASTBOUND)			N/A (WESTBOUND)		
	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT	LEFT	THRU	RIGHT
6AM	3	355	0	0	172	9	11	0	5	0	0	0
7AM	10	576	0	0	245	33	5	0	7	0	0	0
8AM	11	443	0	0	279	9	14	0	10	0	0	0
11AM	2	272	0	0	292	10	15	0	13	0	0	0
12PM	5	359	0	0	331	7	14	0	10	0	0	0
1PM	1	325	0	0	327	14	12	0	5	0	0	0
2PM	6	354	0	0	414	6	14	0	5	0	0	0
3PM	2	363	0	0	516	5	20	0	11	0	0	0
4PM	3	407	0	0	464	15	26	0	12	0	0	0
5PM	5	397	0	0	506	12	18	0	5	0	0	0

EXISTING TRAFFIC VOLUMES - SR 149							TOTAL (Both Approaches)
MAJOR							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	3	355	0	0	172	9	539
7:00 AM	10	576	0	0	245	33	864
8:00 AM	11	443	0	0	279	9	742
11:00 AM	2	272	0	0	292	10	576
12:00 PM	5	359	0	0	331	7	702
1:00 PM	1	325	0	0	327	14	667
2:00 PM	6	354	0	0	414	6	779
3:00 PM	2	363	0	0	516	5	886
4:00 PM	3	407	0	0	464	15	889
5:00 PM	5	397	0	0	506	12	920

2023 NO-BUILD TRAFFIC VOLUMES - SR 149 (GROWTH FACTOR = 1.005)							TOTAL (Both Approaches)
MAJOR							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	3	357	0	0	173	9	542
7:00 AM	10	579	0	0	246	33	869
8:00 AM	11	445	0	0	281	9	746
11:00 AM	2	274	0	0	293	10	579
12:00 PM	5	361	0	0	333	7	705
1:00 PM	1	327	0	0	328	14	670
2:00 PM	6	355	0	0	416	6	782
3:00 PM	2	365	0	0	519	5	890
4:00 PM	3	409	0	0	467	15	894
5:00 PM	5	399	0	0	509	12	924

2043 NO-BUILD TRAFFIC VOLUMES - SR 149 (GROWTH FACTOR = 1.105)							TOTAL (Both Approaches)
MAJOR							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	4	392	0	0	190	10	596
7:00 AM	11	637	0	0	271	36	955
8:00 AM	12	489	0	0	308	10	820
11:00 AM	2	301	0	0	322	11	637
12:00 PM	5	397	0	0	366	7	775
1:00 PM	1	360	0	0	361	15	737
2:00 PM	6	391	0	0	457	6	860
3:00 PM	2	401	0	0	571	5	979
4:00 PM	4	450	0	0	513	16	983
5:00 PM	5	438	0	0	559	14	1016

EXISTING TRAFFIC VOLUMES - RECO DRIVE							TOTAL (Highest Volume Approach)
MINOR							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	11	0	5	0	0	0	16
7:00 AM	5	0	7	0	0	0	12
8:00 AM	14	0	10	0	0	0	24
11:00 AM	15	0	13	0	0	0	28
12:00 PM	14	0	10	0	0	0	24
1:00 PM	12	0	5	0	0	0	17
2:00 PM	14	0	5	0	0	0	18
3:00 PM	20	0	11	0	0	0	31
4:00 PM	26	0	12	0	0	0	38
5:00 PM	18	0	5	0	0	0	23

2023 NO-BUILD TRAFFIC VOLUMES - RECO DRIVE (GROWTH FACTOR = 1.005)							TOTAL (Highest Volume Approach)
MINOR							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	11	0	5	0	0	0	16
7:00 AM	5	0	7	0	0	0	12
8:00 AM	14	0	10	0	0	0	24
11:00 AM	15	0	13	0	0	0	28
12:00 PM	14	0	10	0	0	0	24
1:00 PM	12	0	5	0	0	0	17
2:00 PM	14	0	5	0	0	0	18
3:00 PM	20	0	11	0	0	0	31
4:00 PM	26	0	12	0	0	0	38
5:00 PM	18	0	5	0	0	0	23

2043 NO-BUILD TRAFFIC VOLUMES - RECO DRIVE (GROWTH FACTOR = 1.105)							TOTAL (Highest Volume Approach)
MINOR							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	12	0	6	0	0	0	18
7:00 AM	6	0	8	0	0	0	13
8:00 AM	15	0	11	0	0	0	27
11:00 AM	17	0	14	0	0	0	31
12:00 PM	15	0	11	0	0	0	27
1:00 PM	14	0	5	0	0	0	19
2:00 PM	15	0	5	0	0	0	20
3:00 PM	22	0	12	0	0	0	34
4:00 PM	29	0	13	0	0	0	42
5:00 PM	20	0	6	0	0	0	25

MINOR STREET RIGHT-TURN REDUCTION CALCULATIONS - 2023 BUILD								
	Right Turns	Approach	% RT out of Approach	Mainline Volume per lane	Base Right Turn Reduction	Mainline Congestion Factor	Adjusted Right Turns	Adj. Total Highest Vol. Approach
6:00 AM	5	16	31%	182	20%	0%	4	15
7:00 AM	7	12	58%	279	40%	0%	4	9
8:00 AM	10	24	42%	290	40%	0%	6	20
11:00 AM	13	28	46%	303	40%	0%	8	23
12:00 PM	10	24	42%	340	40%	0%	6	20
1:00 PM	5	17	27%	342	20%	0%	4	16
2:00 PM	5	18	25%	421	20%	5%	4	17
3:00 PM	11	31	35%	524	20%	10%	10	30
4:00 PM	12	38	32%	482	20%	5%	10	36
5:00 PM	5	23	22%	521	20%	10%	5	23

MINOR STREET RIGHT-TURN REDUCTION CALCULATIONS - 2023 BUILD								
	Right Turns	Approach	% RT out of Approach	Mainline Volume per lane	Base Right Turn Reduction	Mainline Congestion Factor	Adjusted Right Turns	Adj. Total Highest Vol. Approach
6:00 AM	6	18	31%	200	20%	0%	4	17
7:00 AM	8	13	58%	307	40%	0%	5	10
8:00 AM	11	27	42%	318	40%	0%	7	22
11:00 AM	14	31	46%	333	40%	0%	9	25
12:00 PM	11	27	42%	373	40%	0%	7	22
1:00 PM	5	19	27%	376	20%	0%	4	18
2:00 PM	5	20	25%	463	20%	5%	4	19
3:00 PM	12	34	35%	576	20%	10%	11	33
4:00 PM	13	42	32%	529	20%	10%	12	41
5:00 PM	6	25	22%	573	20%	10%	5	25

LOVE'S DEVELOPMENT GASOLINE SERVICE STATION W/ CONVENIENCE MARKET (ITE 945) 16 VEHICLE FUELING POSITIONS

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	2057	2057	4114	494	494	1563	1563
6:00 AM	4.7%	4.5%	4.7%	4.5%	97	93	189	23	22	73	70
7:00 AM	6.0%	5.9%	6.0%	5.9%	123	121	245	30	29	94	92
8:00 AM	6.5%	6.4%	6.5%	6.4%	134	132	265	32	32	102	100
11:00 AM	5.8%	5.7%	5.8%	5.7%	119	117	237	29	28	91	89
12:00 PM	6.6%	6.6%	6.6%	6.6%	136	136	272	33	33	103	103
1:00 PM	6.2%	5.9%	6.2%	5.9%	128	121	249	31	29	97	92
2:00 PM	6.0%	6.2%	6.0%	6.2%	123	128	251	30	31	94	97
3:00 PM	6.8%	6.8%	6.8%	6.8%	140	140	280	34	34	106	106
4:00 PM	6.3%	6.5%	6.3%	6.5%	130	134	263	31	32	98	102
5:00 PM	6.7%	6.9%	6.7%	6.9%	138	142	280	33	34	105	108

LOVE'S DEVELOPMENT FAST FOOD RESTAURANT W/ DRIVE-THROUGH WINDOW (ITE 934) 3,100 S.F.

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	725	725	1450	341	341	384	384
6:00 AM	2.3%	1.9%	2.3%	1.9%	17	14	30	8	6	9	7
7:00 AM	3.4%	3.1%	3.4%	3.1%	25	22	47	12	11	13	12
8:00 AM	3.5%	3.4%	3.5%	3.4%	25	25	50	12	12	13	13
11:00 AM	9.1%	7.7%	9.1%	7.7%	66	56	122	31	26	35	30
12:00 PM	11.9%	12.0%	11.9%	12.0%	86	87	173	41	41	46	46
1:00 PM	7.9%	8.7%	7.9%	8.7%	57	63	120	27	30	30	33
2:00 PM	5.9%	6.5%	5.9%	6.5%	43	47	90	20	22	23	25
3:00 PM	5.7%	5.7%	5.7%	5.7%	41	41	83	19	19	22	22
4:00 PM	5.9%	5.6%	5.9%	5.6%	43	41	83	20	19	23	22
5:00 PM	6.9%	6.5%	6.9%	6.5%	50	47	97	24	22	27	25

LOVE'S DEVELOPMENT FAST FOOD RESTAURANT WITHOUT DRIVE-THROUGH WINDOW (ITE 933) 2,900 S.F.

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	653	653	1306	653	653	0	0
6:00 AM	1.4%	0.7%	0.0%	0.0%	9	5	14	9	5	0	0
7:00 AM	1.9%	1.7%	0.0%	0.0%	12	11	24	12	11	0	0
8:00 AM	1.8%	2.1%	0.0%	0.0%	12	14	25	12	14	0	0
11:00 AM	12.2%	9.6%	0.0%	0.0%	80	63	142	80	63	0	0
12:00 PM	14.2%	14.1%	0.0%	0.0%	93	92	185	93	92	0	0
1:00 PM	10.5%	10.2%	0.0%	0.0%	69	67	135	69	67	0	0
2:00 PM	7.9%	8.1%	0.0%	0.0%	52	53	104	52	53	0	0
3:00 PM	9.6%	9.1%	0.0%	0.0%	63	59	122	63	59	0	0
4:00 PM	6.5%	6.3%	0.0%	0.0%	42	41	84	42	41	0	0
5:00 PM	5.6%	6.8%	0.0%	0.0%	37	44	81	37	44	0	0

LOVE'S DEVELOPMENT PASSENGER CAR GENERATED TRIPS

	Total primary		Total pass-by	
	inbound	outbound	inbound	outbound
	6:00 AM	40	33	82
7:00 AM	54	51	107	104
8:00 AM	56	57	115	113
11:00 AM	139	117	126	119
12:00 PM	166	166	149	149
1:00 PM	126	125	127	126
2:00 PM	101	106	116	122
3:00 PM	116	112	128	128
4:00 PM	94	92	121	123
5:00 PM	93	101	131	133

LOVE'S DEVELOPMENT PASSENGER CAR GENERATED TRIPS W/ INTERNAL CAPTURE

	Total primary		Total pass-by	
	inbound	outbound	inbound	outbound
	6:00 AM	35	29	72
7:00 AM	47	44	93	91
8:00 AM	49	49	100	98
11:00 AM	121	102	109	103
12:00 PM	134	134	121	121
1:00 PM	102	102	103	102
2:00 PM	82	86	94	99
3:00 PM	94	91	104	104
4:00 PM	76	75	98	100
5:00 PM	75	81	106	108

LOVE'S DEVELOPMENT TRUCK STOP (ITE 950) 9 TRUCK FUELING POSITIONS

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	1008	1008	2016	1008	1008	0	0
6:00 AM	3.7%	5.1%	0.0%	0.0%	37	51	89	37	51	0	0
7:00 AM	3.3%	4.9%	0.0%	0.0%	33	49	83	33	49	0	0
8:00 AM	5.5%	5.7%	0.0%	0.0%	55	57	113	55	57	0	0
11:00 AM	5.8%	5.0%	0.0%	0.0%	58	50	109	58	50	0	0
12:00 PM	6.3%	6.0%	0.0%	0.0%	64	60	124	64	60	0	0
1:00 PM	5.7%	5.4%	0.0%	0.0%	57	54	112	57	54	0	0
2:00 PM	4.4%	4.4%	0.0%	0.0%	44	44	89	44	44	0	0
3:00 PM	5.6%	4.6%	0.0%	0.0%	56	46	103	56	46	0	0
4:00 PM	5.8%	4.6%	0.0%	0.0%	58	46	105	58	46	0	0
5:00 PM	5.8%	5.8%	0.0%	0.0%	58	58	117	58	58	0	0

LOVE'S DEVELOPMENT TIRE STORE (ITE 848) 3 SERVICE BAYS

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	50	50	100	50	50	0	0
6:00 AM	0.7%	0.0%	0.0%	0.0%	0	0	0	0	0	0	0
7:00 AM	4.0%	0.8%	0.0%	0.0%	2	0	2	2	0	0	0
8:00 AM	9.0%	4.2%	0.0%	0.0%	5	2	7	5	2	0	0
11:00 AM	9.1%	8.7%	0.0%	0.0%	5	4	9	5	4	0	0
12:00 PM	6.8%	8.7%	0.0%	0.0%	3	4	8	3	4	0	0
1:00 PM	13.1%	11.7%	0.0%	0.0%	7	6	12	7	6	0	0
2:00 PM	9.5%	11.4%	0.0%	0.0%	5	6	10	5	6	0	0
3:00 PM	9.1%	10.2%	0.0%	0.0%	5	5	10	5	5	0	0
4:00 PM	8.5%	9.7%	0.0%	0.0%	4	5	9	4	5	0	0
5:00 PM	5.3%	7.7%	0.0%	0.0%	3	4	7	3	4	0	0

LOVE'S DEVELOPMENT CAMPGROUND/RECREATIONAL VEHICLE PARK (ITE 416) 70 CAMPSITES

	PRIMARY	PRIMARY	PASS-BY	PASS-BY	TOTAL TRIPS			PRIMARY TRIPS		PASS BY TRIPS	
	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Hourly Percentage Distribution	Inbound	Outbound	Total	Inbound	Outbound	Inbound	Outbound
	Inbound	Outbound	Inbound	Outbound	95	95	190	95	95	0	0
6:00 AM	4.7%	4.5%	0.0%	0.0%	4	4	9	4	4	0	0
7:00 AM	6.0%	5.9%	0.0%	0.0%	3	6	9	3	6	0	0
8:00 AM	6.5%	6.4%	0.0%	0.0%	3	6	9	3	6	0	0
11:00 AM	5.8%	5.7%	0.0%	0.0%	3	5	8	3	5	0	0
12:00 PM	6.6%	6.6%	0.0%	0.0%	3	6	10	3	6	0	0
1:00 PM	6.2%	5.9%	0.0%	0.0%	3	6	9	3	6	0	0
2:00 PM	6.0%	6.2%	0.0%	0.0%	3	6	9	3	6	0	0
3:00 PM	6.8%	6.8%	0.0%	0.0%	3	6	10	3	6	0	0
4:00 PM	6.3%	6.5%	0.0%	0.0%	3	6	9	3	6	0	0
5:00 PM	6.7%	6.9%	0.0%	0.0%	3	7	10	3	7	0	0

LOVE'S DEVELOPMENT TRUCK GENERATED TRIPS

	Total primary		Total pass-by	
	inbound	outbound	inbound	outbound
	6:00 AM	42	56	0
7:00 AM	38	55	0	0
8:00 AM	63	66	0	0
11:00 AM	66	60	0	0
12:00 PM	70	71	0	0
1:00 PM	67	66	0	0
2:00 PM	52	56	0	0
3:00 PM	64	58	0	0
4:00 PM	66	57	0	0
5:00 PM	64	69	0	0

LOVE'S DEVELOPMENT TRUCK GENERATED TRIPS W/ INTERNAL CAPTURE

	Total primary		Total pass-by	
	inbound	outbound	inbound	outbound
	6:00 AM	37	48	0
7:00 AM	33	48	0	0
8:00 AM	55	57	0	0
11:00 AM	57	52	0	0
12:00 PM	57	58	0	0
1:00 PM	54	53	0	0
2:00 PM	42	45	0	0
3:00 PM	52	47	0	0
4:00 PM	53	46	0	0
5:00 PM	52	56	0	0

PRIMARY DIRECTIONAL DISTRIBUTION AT INTERSECTION (PASSENGER CAR)												
In/Out →	SR 149			SR 149			RECO DRIVE			N/A		
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
	Inbound					Inbound	Outbound		Outbound			
AM	45%					55%	55%		45%			
PM	30%					70%	70%		30%			

PASS-BY DIRECTIONAL DISTRIBUTION AT INTERSECTION (PASSENGER CAR)												
In/Out →	SR 149			SR 149			RECO DRIVE			N/A		
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
	Inbound	Inbound			Inbound	Inbound	Outbound		Outbound			
AM	25%	-25%			-25%	75%	75%		25%			
PM	15%	-15%			-20%	85%	80%		20%			

PRIMARY DIRECTIONAL DISTRIBUTION AT INTERSECTION (TRUCK)												
In/Out →	SR 149			SR 149			RECO DRIVE			N/A		
	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR
		Outbound			Inbound							
AM		75%			75%							
PM		85%			85%							

LOVE'S DEVELOPMENT GENERATED TRAFFIC VOLUMES - SR 149							TOTAL (Both Approaches)
MAJOR STREET							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	34	18			10	73	135
7:00 AM	44	13			2	95	154
8:00 AM	47	18			16	102	183
11:00 AM	82	12			16	149	258
12:00 PM	58	31			24	197	310
1:00 PM	46	30			26	159	261
2:00 PM	39	24			17	138	218
3:00 PM	44	24			24	154	245
4:00 PM	37	25			26	137	225
5:00 PM	39	31			23	143	236

2023 BUILD TRAFFIC VOLUMES - SR 149							TOTAL (Both Approaches)
MAJOR							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	37	375	0	0	182	82	676
7:00 AM	54	592	0	0	248	128	1023
8:00 AM	58	463	0	0	297	111	929
11:00 AM	84	286	0	0	309	159	837
12:00 PM	63	392	0	0	357	203	1015
1:00 PM	47	357	0	0	354	173	931
2:00 PM	44	380	0	0	433	143	1000
3:00 PM	46	389	0	0	543	158	1136
4:00 PM	41	434	0	0	492	151	1118
5:00 PM	43	430	0	0	532	156	1161

2043 BUILD TRAFFIC VOLUMES - SR 149							TOTAL (Both Approaches)
MAJOR							
NBL	NBT	NBR	SBL	SBT	SBR		
6:00 AM	37	411	0	0	199	83	730
7:00 AM	55	650	0	0	273	132	1109
8:00 AM	59	507	0	0	325	112	1003
11:00 AM	84	313	0	0	338	160	895
12:00 PM	63	428	0	0	390	204	1085
1:00 PM	47	390	0	0	386	174	997
2:00 PM	45	415	0	0	474	144	1078
3:00 PM	46	425	0	0	594	159	1224
4:00 PM	41	474	0	0	539	153	1207
5:00 PM	44	470	0	0	583	157	1253

LOVE'S DEVELOPMENT GENERATED TRAFFIC VOLUMES - RECO DRIVE							TOTAL (Highest Volume Approach)
MINOR STREET							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	67		30				96
7:00 AM	92		43				135
8:00 AM	101		47				148
11:00 AM	133		72				205
12:00 PM	191		64				255
1:00 PM	153		51				203
2:00 PM	139		45				184
3:00 PM	147		48				195
4:00 PM	132		42				175
5:00 PM	143		46				189

2023 BUILD TRAFFIC VOLUMES - RECO DRIVE							TOTAL (Highest Volume Approach)
MINOR							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	78	0	35	0	0	0	113
7:00 AM	97	0	50	0	0	0	147
8:00 AM	115	0	57	0	0	0	172
11:00 AM	149	0	85	0	0	0	233
12:00 PM	205	0	74	0	0	0	279
1:00 PM	165	0	55	0	0	0	220
2:00 PM	153	0	50	0	0	0	202
3:00 PM	167	0	59	0	0	0	226
4:00 PM	158	0	54	0	0	0	213
5:00 PM	161	0	51	0	0	0	212

2043 BUILD TRAFFIC VOLUMES - RECO DRIVE							TOTAL (Highest Volume Approach)
MINOR							
	EBL	EBT	EBR	WBL	WBT	WBR	
6:00 AM	79	0	35	0	0	0	114
7:00 AM	98	0	50	0	0	0	148
8:00 AM	117	0	58	0	0	0	174
11:00 AM	150	0	86	0	0	0	236
12:00 PM	206	0	75	0	0	0	282
1:00 PM	166	0	56	0	0	0	222
2:00 PM	154	0	50	0	0	0	204
3:00 PM	169	0	60	0	0	0	229
4:00 PM	161	0	56	0	0	0	216
5:00 PM	163	0	51	0	0	0	215

MINOR STREET RIGHT-TURN REDUCTION CALCULATIONS - 2023 BUILD								
	Right Turns	Approach	% RT out of Approach	Mainline Volume per lane	Base Right Turn Reduction	Mainline Congestion Factor	Adjusted Right Turns	Adj. Total Highest Vol. Approach
6:00 AM	35	113	31%	182	20%	0%	28	106
7:00 AM	50	147	34%	248	20%	0%	40	137
8:00 AM	57	172	33%	297	20%	0%	46	161
11:00 AM	85	233	36%	309	40%	0%	51	199
12:00 PM	74	279	27%	357	20%	0%	60	264
1:00 PM	55	220	25%	354	20%	0%	44	209
2:00 PM	50	202	25%	433	20%	5%	42	195
3:00 PM	59	226	26%	543	20%	10%	53	220
4:00 PM	54	213	26%	492	20%	5%	46	205
5:00 PM	51	212	24%	532	20%	10%	46	207

MINOR STREET RIGHT-TURN REDUCTION CALCULATIONS - 2043 BUILD								
	Right Turns	Approach	% RT out of Approach	Mainline Volume per lane	Base Right Turn Reduction	Mainline Congestion Factor	Adjusted Right Turns	Adj. Total Highest Vol. Approach
6:00 AM	35	114	31%	199	20%	0%	28	107
7:00 AM	50	148	34%	273	20%	0%	40	138
8:00 AM	58	174	33%	325	20%	0%	46	163
11:00 AM	86	236	36%	338	40%	0%	52	202
12:00 PM	75	282	27%	390	20%	0%	60	266
1:00 PM	56	222	25%	386	20%	0%	45	211
2:00 PM	50	204	25%	474	20%	5%	43	197
3:00 PM	60	229	26%	594	20%	10%	54	223
4:00 PM	56	216	26%	539	20%	10%	50	211
5:00 PM	51	215	24%	583	20%	10%	46	209

TRAFFIC SIGNAL WARRANT STUDY TRAFFIC VOLUMES								
2023 NO-BUILD		2023 BUILD		2043 NO-BUILD		2043 BUILD		
MAJOR	MINOR	MAJOR	MINOR	MAJOR	MINOR	MAJOR	MINOR	
6:00 AM	542	15	676	106	596	17	730	107
7:00 AM	869	9	1023	137	955	10	1109	138
8:00 AM	746	20	929	161	820	22	1003	163
11:00 AM	579	23	837	199	637	25	895	202
12:00 PM	705	20	1015	264	775	22	1085	266
1:00 PM	670	16	931	209	737	18	997	211
2:00 PM	782	17	1000	195	860	19	1078	197
3:00 PM	890	30	1136	220	979	33	1224	223
4:00 PM	894	36	1118	205	983	41	1207	211
5:00 PM	924	23	1161	207	1016	25	1253	209

**APPENDIX B
EXISTING TRAFFIC COUNT DATA**

SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	WB I 70 On Ramp Eastbound					WB I-70 Off Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 7:00AM	0	0	0	0	0	22	0	49	0	71	12	93	0	0	105	0	53	5	0	58	234
7:15AM	0	0	0	0	0	29	0	32	0	61	18	58	0	0	76	0	84	10	0	94	231
7:30AM	0	0	0	0	0	24	0	25	0	49	12	41	0	0	53	0	35	11	0	46	148
7:45AM	0	0	0	0	0	34	0	25	0	59	9	29	0	0	38	0	30	17	0	47	144
Hourly Total	0	0	0	0	0	109	0	131	0	240	51	221	0	0	272	0	202	43	0	245	757
8:00AM	0	0	0	0	0	27	0	14	0	41	13	26	0	0	39	0	36	6	0	42	122
8:15AM	0	0	0	0	0	29	0	33	0	62	8	66	0	0	74	0	44	8	0	52	188
8:30AM	0	0	0	0	0	28	1	22	0	51	9	63	0	0	72	0	81	15	0	96	219
8:45AM	0	0	0	0	0	22	0	37	0	59	10	44	0	0	54	0	77	17	0	94	207
Hourly Total	0	0	0	0	0	106	1	106	0	213	40	199	0	0	239	0	238	46	0	284	736
4:00PM	0	0	0	0	0	52	0	28	0	80	18	37	0	0	55	0	48	17	0	65	200
4:15PM	0	0	0	0	0	61	0	38	0	99	13	39	0	0	52	0	47	11	0	58	209
4:30PM	0	0	0	0	0	66	1	41	0	108	11	50	0	0	61	0	64	8	0	72	241
4:45PM	0	0	0	0	0	46	0	37	0	83	11	70	0	0	81	0	48	17	0	65	229
Hourly Total	0	0	0	0	0	225	1	144	0	370	53	196	0	0	249	0	207	53	0	260	879
5:00PM	0	0	0	0	0	78	0	48	0	126	16	70	0	0	86	0	58	10	0	68	280
5:15PM	0	0	0	0	0	69	0	49	0	118	10	69	0	0	79	0	75	16	0	91	288
5:30PM	0	0	0	0	0	39	1	31	0	71	9	47	0	0	56	0	48	11	0	59	186
5:45PM	0	0	0	0	0	62	1	38	0	101	13	33	0	0	46	0	31	9	0	40	187
Hourly Total	0	0	0	0	0	248	2	166	0	416	48	219	0	0	267	0	212	46	0	258	941
Total	0	0	0	0	0	688	4	547	0	1239	192	835	0	0	1027	0	859	188	0	1047	3313
% Approach	0%	0%	0%	0%	0%	55.5%	0.3%	44.1%	0%	-	18.7%	81.3%	0%	0%	-	0%	82.0%	18.0%	0%	-	-
% Total	0%	0%	0%	0%	0%	20.8%	0.1%	16.5%	0%	37.4%	5.8%	25.2%	0%	0%	31.0%	0%	25.9%	5.7%	0%	31.6%	-
Lights	0	0	0	0	0	583	0	511	0	1094	104	809	0	0	913	0	817	175	0	992	2999
% Lights	0%	0%	0%	0%	0%	84.7%	0%	93.4%	0%	88.3%	54.2%	96.9%	0%	0%	88.9%	0%	95.1%	93.1%	0%	94.7%	90.5%
Articulated Trucks and Single-Unit Trucks	0	0	0	0	0	102	4	34	0	140	88	8	0	0	96	0	37	13	0	50	286
% Articulated Trucks and Single-Unit Trucks	0%	0%	0%	0%	0%	14.8%	100%	6.2%	0%	11.3%	45.8%	1.0%	0%	0%	9.3%	0%	4.3%	6.9%	0%	4.8%	8.6%
Buses	0	0	0	0	0	3	0	2	0	5	0	18	0	0	18	0	5	0	0	5	28
% Buses	0%	0%	0%	0%	0%	0.4%	0%	0.4%	0%	0.4%	0%	2.2%	0%	0%	1.8%	0%	0.6%	0%	0%	0.5%	0.8%

*L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

[N] SR-149

Total: 2429

In: 1047

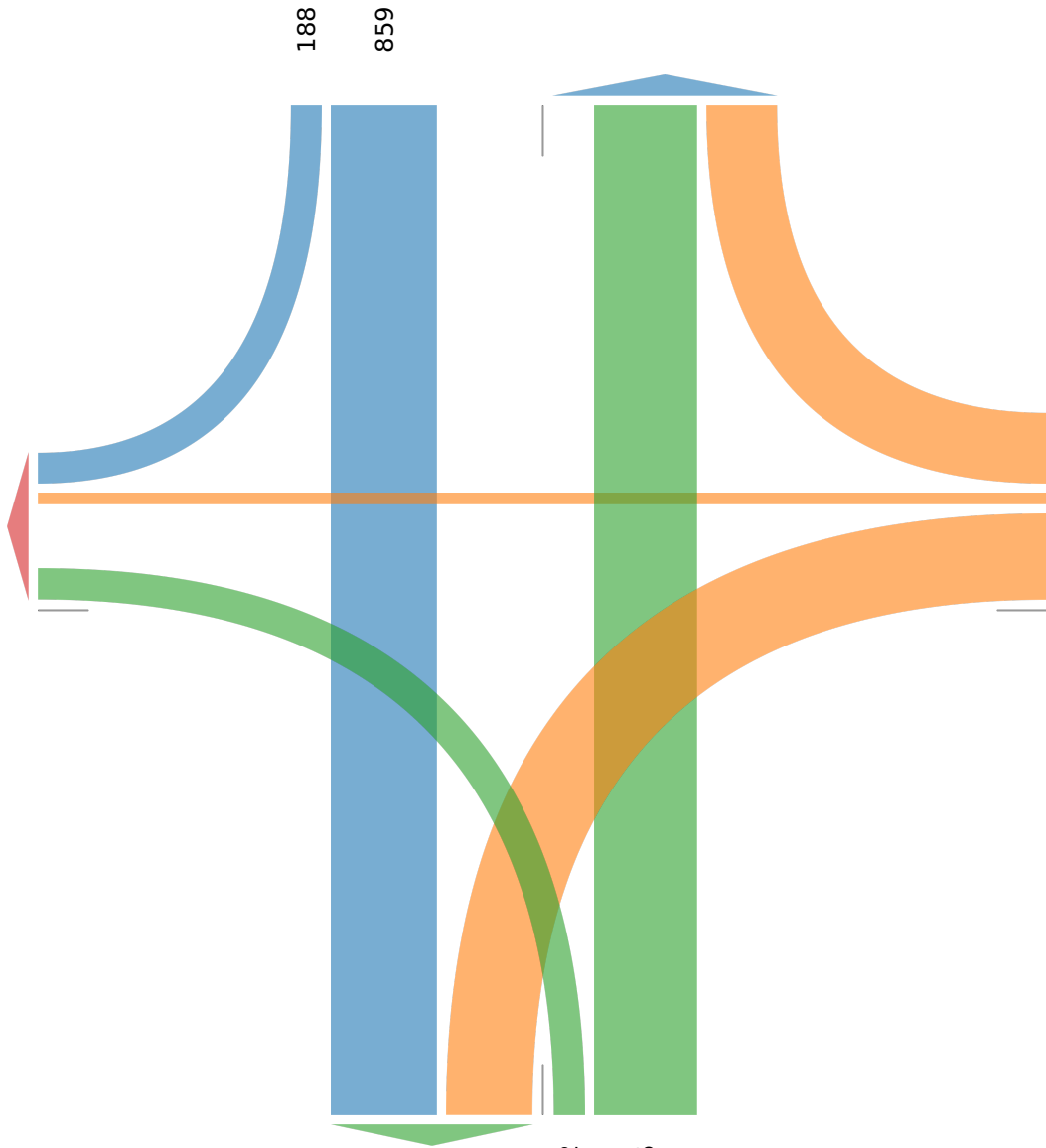
Out: 1382

[W] WB I 70 On Ramp

Total: 384

In: 0

Out: 384



188

859

547

4

688

192

835

Out: 1547

In: 1027

Total: 2574

[S] SR-149

Out: 0 In: 1239

Total: 1239

[E] WB I-70 Off Ramp

SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	WB I 70 On Ramp Eastbound					WB I-70 Off Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 7:00AM	0	0	0	0	0	22	0	49	0	71	12	93	0	0	105	0	53	5	0	58	234
7:15AM	0	0	0	0	0	29	0	32	0	61	18	58	0	0	76	0	84	10	0	94	231
7:30AM	0	0	0	0	0	24	0	25	0	49	12	41	0	0	53	0	35	11	0	46	148
7:45AM	0	0	0	0	0	34	0	25	0	59	9	29	0	0	38	0	30	17	0	47	144
Total	0	0	0	0	0	109	0	131	0	240	51	221	0	0	272	0	202	43	0	245	757
% Approach	0%	0%	0%	0%	-	45.4%	0%	54.6%	0%	-	18.8%	81.3%	0%	0%	-	0%	82.4%	17.6%	0%	-	-
% Total	0%	0%	0%	0%	0%	14.4%	0%	17.3%	0%	31.7%	6.7%	29.2%	0%	0%	35.9%	0%	26.7%	5.7%	0%	32.4%	-
PHF	-	-	-	-	-	0.801	-	0.668	-	0.845	0.708	0.594	-	-	0.648	-	0.601	0.632	-	0.652	0.809
Lights	0	0	0	0	0	89	0	116	0	205	34	216	0	0	250	0	184	39	0	223	678
% Lights	0%	0%	0%	0%	-	81.7%	0%	88.5%	0%	85.4%	66.7%	97.7%	0%	0%	91.9%	0%	91.1%	90.7%	0%	91.0%	89.6%
Articulated Trucks and Single-Unit Trucks	0	0	0	0	0	20	0	15	0	35	17	1	0	0	18	0	14	4	0	18	71
% Articulated Trucks and Single-Unit Trucks	0%	0%	0%	0%	-	18.3%	0%	11.5%	0%	14.6%	33.3%	0.5%	0%	0%	6.6%	0%	6.9%	9.3%	0%	7.3%	9.4%
Buses	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	0	4	0	0	4	8
% Buses	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	0%	1.8%	0%	0%	1.5%	0%	2.0%	0%	0%	1.6%	1.1%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

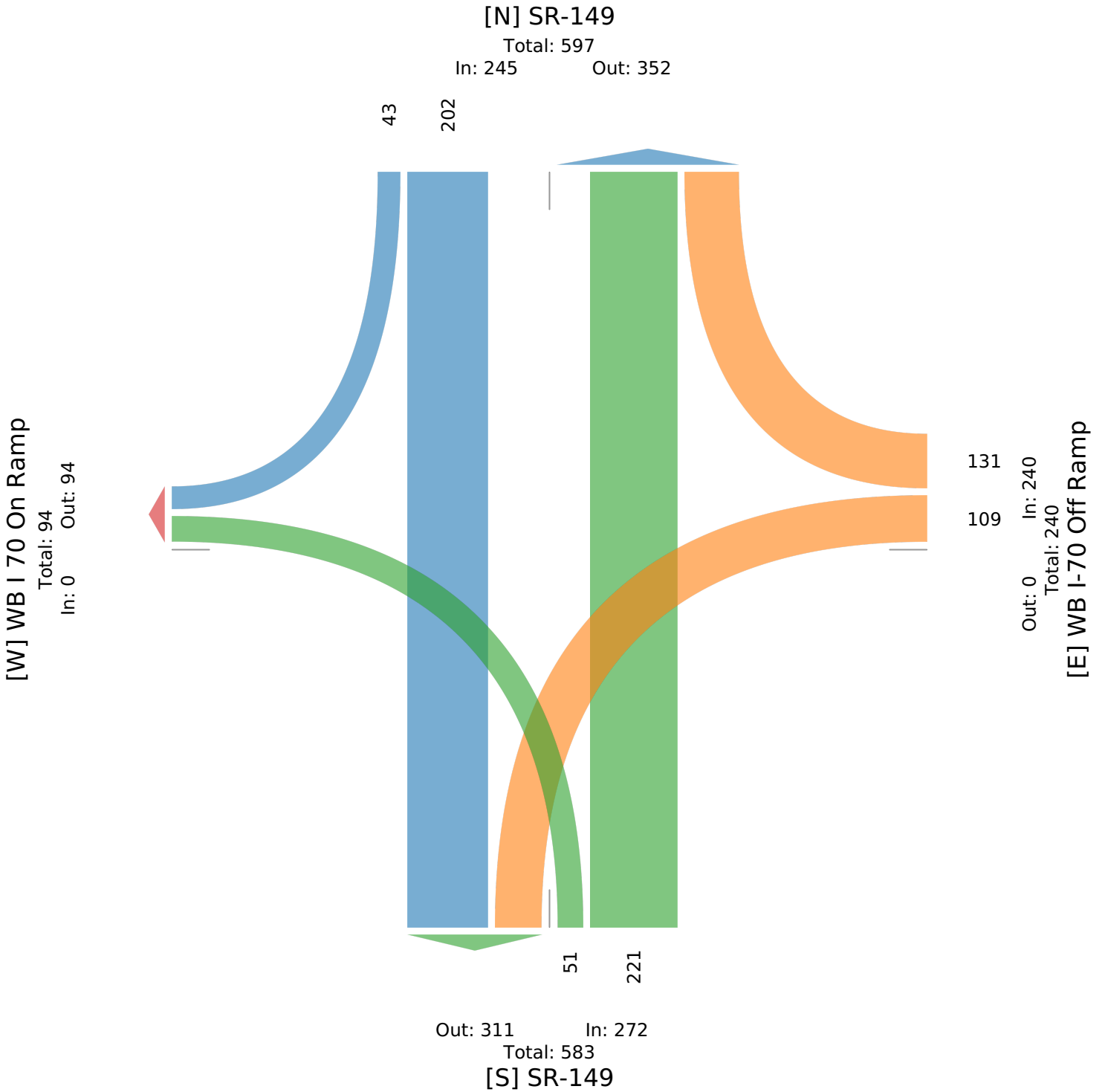
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	WB I 70 On Ramp Eastbound					WB I-70 Off Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 4:30PM	0	0	0	0	0	66	1	41	0	108	11	50	0	0	61	0	64	8	0	72	241
4:45PM	0	0	0	0	0	46	0	37	0	83	11	70	0	0	81	0	48	17	0	65	229
5:00PM	0	0	0	0	0	78	0	48	0	126	16	70	0	0	86	0	58	10	0	68	280
5:15PM	0	0	0	0	0	69	0	49	0	118	10	69	0	0	79	0	75	16	0	91	288
Total	0	0	0	0	0	259	1	175	0	435	48	259	0	0	307	0	245	51	0	296	1038
% Approach	0%	0%	0%	0%	-	59.5%	0.2%	40.2%	0%	-	15.6%	84.4%	0%	0%	-	0%	82.8%	17.2%	0%	-	-
% Total	0%	0%	0%	0%	0%	25.0%	0.1%	16.9%	0%	41.9%	4.6%	25.0%	0%	0%	29.6%	0%	23.6%	4.9%	0%	28.5%	-
PHF	-	-	-	-	-	0.830	0.250	0.893	-	0.863	0.750	0.925	-	-	0.892	-	0.817	0.750	-	0.813	0.901
Lights	0	0	0	0	0	230	0	172	0	402	29	250	0	0	279	0	243	47	0	290	971
% Lights	0%	0%	0%	0%	-	88.8%	0%	98.3%	0%	92.4%	60.4%	96.5%	0%	0%	90.9%	0%	99.2%	92.2%	0%	98.0%	93.5%
Articulated Trucks and Single-Unit Trucks	0	0	0	0	0	29	1	2	0	32	19	3	0	0	22	0	2	4	0	6	60
% Articulated Trucks and Single-Unit Trucks	0%	0%	0%	0%	-	11.2%	100%	1.1%	0%	7.4%	39.6%	1.2%	0%	0%	7.2%	0%	0.8%	7.8%	0%	2.0%	5.8%
Buses	0	0	0	0	0	0	0	1	0	1	0	6	0	0	6	0	0	0	0	0	7
% Buses	0%	0%	0%	0%	-	0%	0%	0.6%	0%	0.2%	0%	2.3%	0%	0%	2.0%	0%	0%	0%	0%	0%	0.7%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 WB On/Off Ramps - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984609, Location: 40.060645, -81.053175



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

[N] SR-149

Total: 730

In: 296

Out: 434

[W] WB I 70 On Ramp

Total: 100

In: 0

Out: 100



Out: 504

In: 307

Total: 811

[S] SR-149

175

1

259

Out: 0

In: 435

Total: 435

[E] WB I-70 Off Ramp

SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	EB I-70 Off Ramp Eastbound					EB I-70 On Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 7:00AM	9	0	17	0	26	0	0	0	0	0	0	96	59	0	155	24	53	0	0	77	258
7:15AM	12	1	13	0	26	0	0	0	0	0	0	62	55	0	117	40	74	0	0	114	257
7:30AM	5	1	8	0	14	0	0	0	0	0	0	50	87	0	137	21	37	0	0	58	209
7:45AM	11	0	13	0	24	0	0	0	0	0	0	28	81	0	109	17	45	0	0	62	195
Hourly Total	37	2	51	0	90	0	0	0	0	0	0	236	282	0	518	102	209	0	0	311	919
8:00AM	5	0	12	0	17	0	0	0	0	0	0	32	68	0	100	21	40	0	0	61	178
8:15AM	10	0	17	0	27	0	0	0	0	0	0	59	60	0	119	20	49	0	0	69	215
8:30AM	9	0	12	0	21	0	0	0	0	0	0	70	50	0	120	47	67	0	0	114	255
8:45AM	8	0	10	0	18	0	0	0	0	0	0	44	40	0	84	24	75	0	0	99	201
Hourly Total	32	0	51	0	83	0	0	0	0	0	0	205	218	0	423	112	231	0	0	343	849
4:00PM	7	0	15	0	22	0	0	0	0	0	0	48	48	0	96	19	85	0	0	104	222
4:15PM	11	0	27	0	38	0	0	0	0	0	0	43	50	0	93	11	96	0	0	107	238
4:30PM	4	1	20	0	25	0	0	0	0	0	0	57	48	0	105	19	105	0	0	124	254
4:45PM	13	2	24	0	39	0	0	0	0	0	0	67	48	0	115	14	85	0	0	99	253
Hourly Total	35	3	86	0	124	0	0	0	0	0	0	215	194	0	409	63	371	0	0	434	967
5:00PM	14	0	19	0	33	0	0	0	0	0	0	71	52	0	123	18	118	0	0	136	292
5:15PM	16	0	22	0	38	0	0	0	0	0	0	64	46	0	110	21	125	0	0	146	294
5:30PM	10	0	27	0	37	0	0	0	0	0	0	45	38	0	83	15	73	0	0	88	208
5:45PM	9	0	16	0	25	0	0	0	0	0	0	39	40	0	79	13	78	0	0	91	195
Hourly Total	49	0	84	0	133	0	0	0	0	0	0	219	176	0	395	67	394	0	0	461	989
Total	153	5	272	0	430	0	0	0	0	0	0	875	870	0	1745	344	1205	0	0	1549	3724
% Approach	35.6%	1.2%	63.3%	0%	-	0%	0%	0%	0%	-	0%	50.1%	49.9%	0%	-	22.2%	77.8%	0%	0%	-	-
% Total	4.1%	0.1%	7.3%	0%	11.5%	0%	0%	0%	0%	0%	0%	23.5%	23.4%	0%	46.9%	9.2%	32.4%	0%	0%	41.6%	-
Lights	149	1	146	0	296	0	0	0	0	0	0	765	762	0	1527	320	1081	0	0	1401	3224
% Lights	97.4%	20.0%	53.7%	0%	68.8%	0%	0%	0%	0%	-	0%	87.4%	87.6%	0%	87.5%	93.0%	89.7%	0%	0%	90.4%	86.6%
Articulated Trucks and Single-Unit Trucks	3	4	126	0	133	0	0	0	0	0	0	92	106	0	198	24	117	0	0	141	472
% Articulated Trucks and Single-Unit Trucks	2.0%	80.0%	46.3%	0%	30.9%	0%	0%	0%	0%	-	0%	10.5%	12.2%	0%	11.3%	7.0%	9.7%	0%	0%	9.1%	12.7%
Buses	1	0	0	0	1	0	0	0	0	0	0	18	2	0	20	0	7	0	0	7	28
% Buses	0.7%	0%	0%	0%	0.2%	0%	0%	0%	0%	-	0%	2.1%	0.2%	0%	1.1%	0%	0.6%	0%	0%	0.5%	0.8%

*L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

Full Length (7 AM-9 AM, 4 PM-6 PM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

[N] SR-149

Total: 2577

In: 1549 Out: 1028

1205

344

[W] EB I-70 Off Ramp

Total: 430
In: 430 Out: 0

153
5
272

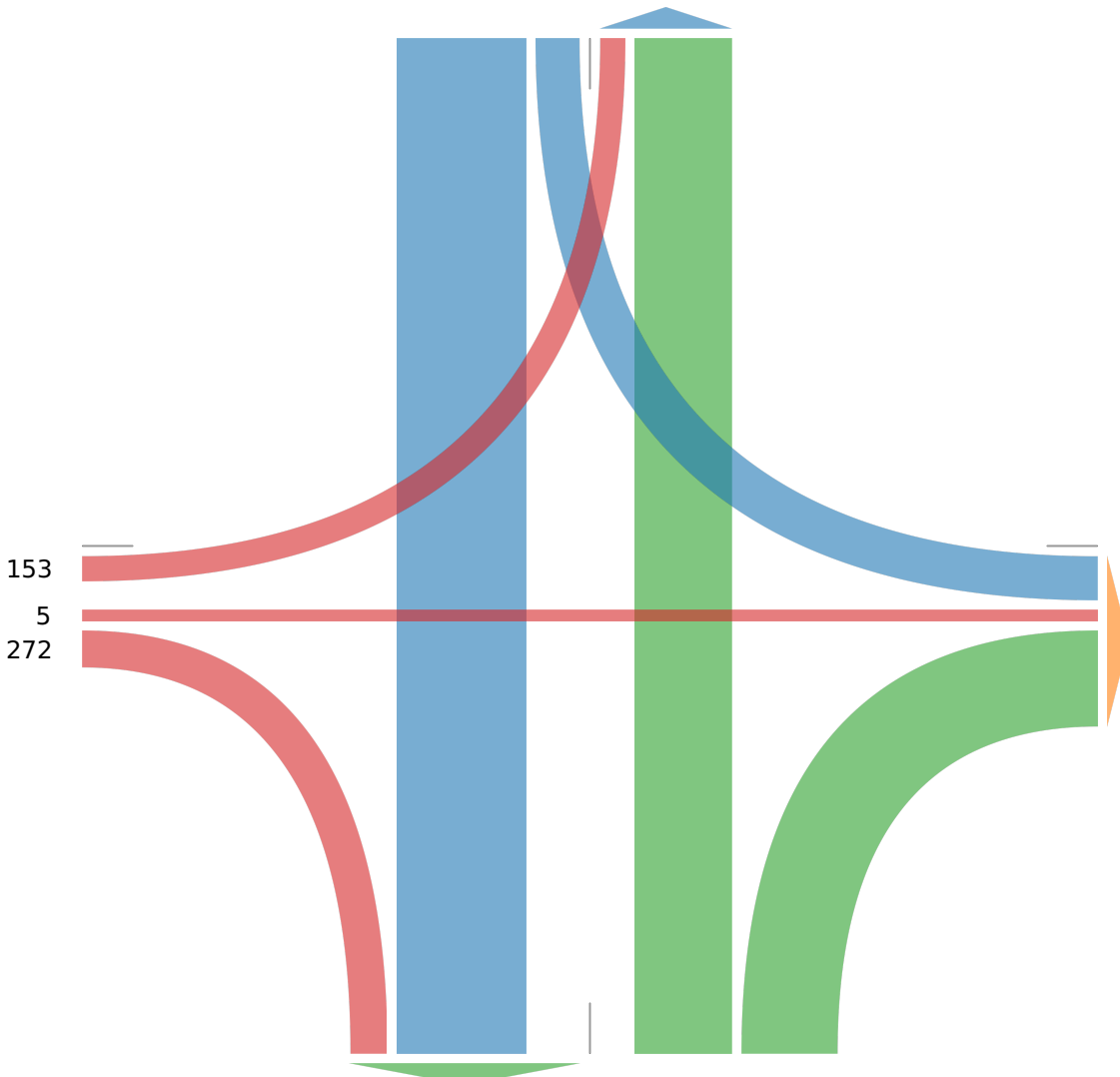
Out: 1219 In: 0
Total: 1219

[E] EB I-70 On Ramp

Out: 1477 In: 1745
Total: 3222
[S] SR-149

875

870



SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	EB I-70 Off Ramp Eastbound					EB I-70 On Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 7:00AM	9	0	17	0	26	0	0	0	0	0	0	96	59	0	155	24	53	0	0	77	258
7:15AM	12	1	13	0	26	0	0	0	0	0	0	62	55	0	117	40	74	0	0	114	257
7:30AM	5	1	8	0	14	0	0	0	0	0	0	50	87	0	137	21	37	0	0	58	209
7:45AM	11	0	13	0	24	0	0	0	0	0	0	28	81	0	109	17	45	0	0	62	195
Total	37	2	51	0	90	0	0	0	0	0	0	236	282	0	518	102	209	0	0	311	919
% Approach	41.1%	2.2%	56.7%	0%	-	0%	0%	0%	0%	-	0%	45.6%	54.4%	0%	-	32.8%	67.2%	0%	0%	-	-
% Total	4.0%	0.2%	5.5%	0%	9.8%	0%	0%	0%	0%	0%	0%	25.7%	30.7%	0%	56.4%	11.1%	22.7%	0%	0%	33.8%	-
PHF	0.771	0.500	0.750	-	0.865	-	-	-	-	-	-	0.615	0.810	-	0.835	0.638	0.706	-	-	0.682	0.891
Lights	37	0	35	0	72	0	0	0	0	0	0	215	260	0	475	92	180	0	0	272	819
% Lights	100%	0%	68.6%	0%	80.0%	0%	0%	0%	0%	-	0%	91.1%	92.2%	0%	91.7%	90.2%	86.1%	0%	0%	87.5%	89.1%
Articulated Trucks and Single-Unit Trucks	0	2	16	0	18	0	0	0	0	0	0	17	20	0	37	10	26	0	0	36	91
% Articulated Trucks and Single-Unit Trucks	0%	100%	31.4%	0%	20.0%	0%	0%	0%	0%	-	0%	7.2%	7.1%	0%	7.1%	9.8%	12.4%	0%	0%	11.6%	9.9%
Buses	0	0	0	0	0	0	0	0	0	0	0	4	2	0	6	0	3	0	0	3	9
% Buses	0%	0%	0%	0%	0%	0%	0%	0%	0%	-	0%	1.7%	0.7%	0%	1.2%	0%	1.4%	0%	0%	1.0%	1.0%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

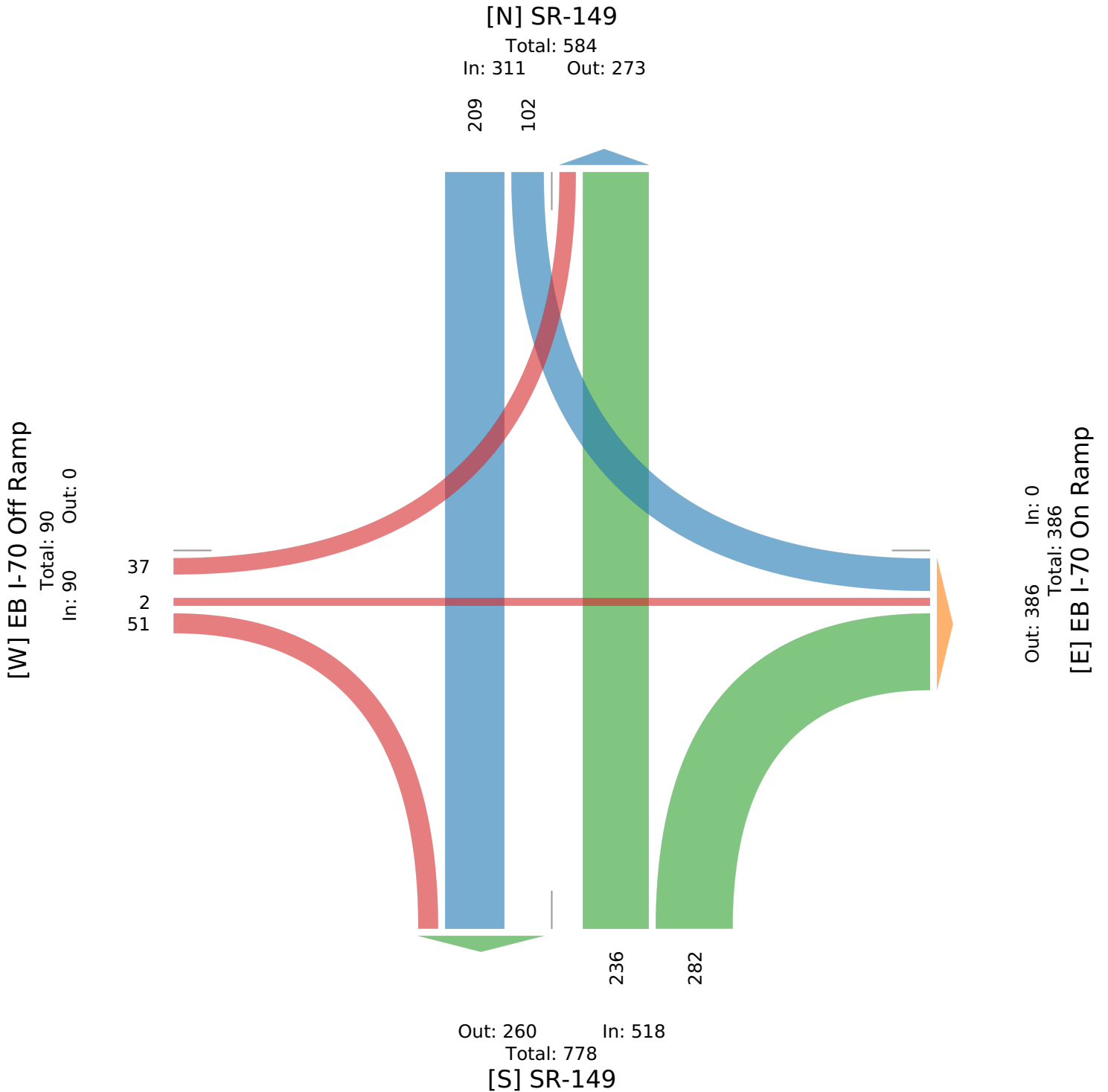
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	EB I-70 Off Ramp Eastbound					EB I-70 On Ramp Westbound					SR-149 Northbound					SR-149 Southbound					Int
	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	L	T	R	U	App	
2022-09-13 4:30PM	4	1	20	0	25	0	0	0	0	0	0	57	48	0	105	19	105	0	0	124	254
4:45PM	13	2	24	0	39	0	0	0	0	0	0	67	48	0	115	14	85	0	0	99	253
5:00PM	14	0	19	0	33	0	0	0	0	0	0	71	52	0	123	18	118	0	0	136	292
5:15PM	16	0	22	0	38	0	0	0	0	0	0	64	46	0	110	21	125	0	0	146	294
Total	47	3	85	0	135	0	0	0	0	0	0	259	194	0	453	72	433	0	0	505	1093
% Approach	34.8%	2.2%	63.0%	0%	-	0%	0%	0%	0%	-	0%	57.2%	42.8%	0%	-	14.3%	85.7%	0%	0%	-	-
% Total	4.3%	0.3%	7.8%	0%	12.4%	0%	0%	0%	0%	0%	0%	23.7%	17.7%	0%	41.4%	6.6%	39.6%	0%	0%	46.2%	-
PHF	0.734	0.375	0.885	-	0.865	-	-	-	-	-	-	0.912	0.933	-	0.921	0.857	0.866	-	-	0.865	0.929
Lights	45	1	43	0	89	0	0	0	0	0	0	233	160	0	393	71	403	0	0	474	956
% Lights	95.7%	33.3%	50.6%	0%	65.9%	0%	0%	0%	0%	-	0%	90.0%	82.5%	0%	86.8%	98.6%	93.1%	0%	0%	93.9%	87.5%
Articulated Trucks and Single-Unit Trucks	1	2	42	0	45	0	0	0	0	0	0	21	34	0	55	1	30	0	0	31	131
% Articulated Trucks and Single-Unit Trucks	2.1%	66.7%	49.4%	0%	33.3%	0%	0%	0%	0%	-	0%	8.1%	17.5%	0%	12.1%	1.4%	6.9%	0%	0%	6.1%	12.0%
Buses	1	0	0	0	1	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	6
% Buses	2.1%	0%	0%	0%	0.7%	0%	0%	0%	0%	-	0%	1.9%	0%	0%	1.1%	0%	0%	0%	0%	0%	0.5%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 & I-70 EB On/Off Ramps - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

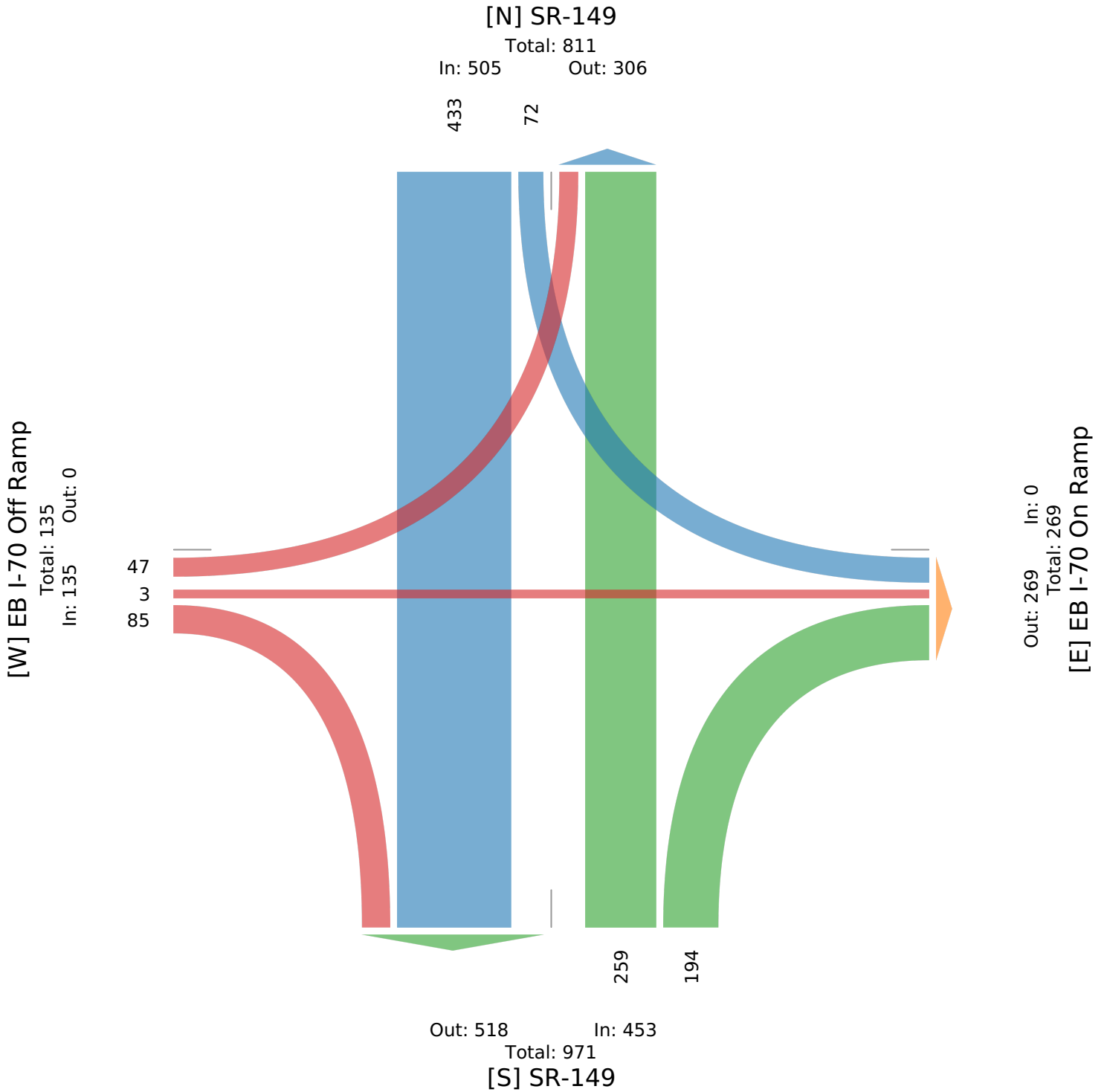
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 984610, Location: 40.059157, -81.052417



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

Full Length ()

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	Reco Eastbound				Belmont Northbound				Belmont Southbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
2022-09-13 6:00AM	0	1	0	1	1	63	0	64	26	2	0	28	93
6:15AM	4	1	0	5	1	77	0	78	35	2	0	37	120
6:30AM	2	0	0	2	0	89	0	89	45	3	0	48	139
6:45AM	5	3	0	8	1	85	0	86	46	1	0	47	141
Hourly Total	11	5	0	16	3	314	0	317	152	8	0	160	493
7:00AM	2	2	0	4	1	147	0	148	58	7	0	65	217
7:15AM	2	1	0	3	5	126	0	131	71	11	0	82	216
7:30AM	1	2	0	3	2	127	0	129	38	4	0	42	174
7:45AM	0	2	0	2	1	110	0	111	50	7	0	57	170
Hourly Total	5	7	0	12	9	510	0	519	217	29	0	246	777
8:00AM	1	2	0	3	2	99	0	101	45	1	0	46	150
8:15AM	3	2	0	5	2	114	0	116	56	1	0	57	178
8:30AM	3	2	0	5	2	107	0	109	71	3	0	74	188
8:45AM	7	4	0	11	4	72	0	76	75	3	0	78	165
Hourly Total	14	10	0	24	10	392	0	402	247	8	0	255	681
11:00AM	4	2	0	6	1	44	0	45	57	3	0	60	111
11:15AM	1	2	0	3	0	56	0	56	63	0	1	64	123
11:30AM	3	2	0	5	0	75	0	75	67	3	0	70	150
11:45AM	7	7	0	14	1	66	0	67	71	3	0	74	155
Hourly Total	15	13	0	28	2	241	0	243	258	9	1	268	539
12:00PM	3	2	0	5	0	83	0	83	63	3	0	66	154
12:15PM	8	3	0	11	1	77	0	78	75	2	0	77	166
12:30PM	2	0	0	2	1	81	0	82	76	0	0	76	160
12:45PM	1	5	0	6	2	77	0	79	79	1	0	80	165
Hourly Total	14	10	0	24	4	318	0	322	293	6	0	299	645
1:00PM	4	3	0	7	0	76	0	76	65	2	0	67	150
1:15PM	5	1	0	6	1	71	0	72	68	5	0	73	151
1:30PM	1	0	0	1	0	74	0	74	75	2	0	77	152
1:45PM	1	0	0	1	0	67	0	67	81	3	0	84	152
Hourly Total	11	4	0	15	1	288	0	289	289	12	0	301	605
2:00PM	5	2	0	7	1	66	0	67	74	0	0	74	148
2:15PM	3	2	0	5	1	83	0	84	133	2	0	135	224
2:30PM	0	0	0	0	2	76	0	78	84	1	0	85	163
2:45PM	4	0	0	4	1	88	0	89	75	2	0	77	170
Hourly Total	12	4	0	16	5	313	0	318	366	5	0	371	705
3:00PM	4	2	0	6	0	74	0	74	128	1	0	129	209
3:15PM	4	4	0	8	1	84	0	85	112	3	0	115	208
3:30PM	9	3	0	12	1	76	0	77	102	0	0	102	191
3:45PM	3	2	0	5	0	87	0	87	115	0	0	115	207
Hourly Total	20	11	0	31	2	321	0	323	457	4	0	461	815
4:00PM	11	7	0	18	0	78	1	79	89	4	0	93	190
4:15PM	7	1	0	8	1	77	0	78	111	7	0	118	204
4:30PM	3	3	0	6	1	100	3	104	109	0	0	109	219
4:45PM	5	1	0	6	1	105	0	106	102	2	0	104	216
Hourly Total	26	12	0	38	3	360	4	367	411	13	0	424	829
5:00PM	11	2	0	13	0	106	0	106	128	3	0	131	250
5:15PM	0	2	0	2	2	102	0	104	137	2	0	139	245
5:30PM	5	1	0	6	1	72	0	73	91	3	0	94	173
5:45PM	2	0	0	2	1	71	0	72	92	3	0	95	169
Hourly Total	18	5	0	23	4	351	0	355	448	11	0	459	837
Total	146	81	0	227	43	3408	4	3455	3138	105	1	3244	6926
% Approach	64.3%	35.7%	0%	-	1.2%	98.6%	0.1%	-	96.7%	3.2%	0%	-	-
% Total	2.1%	1.2%	0%	3.3%	0.6%	49.2%	0.1%	49.9%	45.3%	1.5%	0%	46.8%	-
Lights	121	63	0	184	35	2887	2	2924	2580	88	1	2669	5777

Leg Direction	Reco Eastbound				Belmont Northbound				Belmont Southbound				
Time	L	R	U	App	L	T	U	App	T	R	U	App	Int
% Lights	82.9%	77.8%	0%	81.1%	81.4%	84.7%	50.0%	84.6%	82.2%	83.8%	100%	82.3%	83.4%
Articulated Trucks and Single-Unit Trucks	25	18	0	43	8	493	2	503	536	17	0	553	1099
% Articulated Trucks and Single-Unit Trucks	17.1%	22.2%	0%	18.9%	18.6%	14.5%	50.0%	14.6%	17.1%	16.2%	0%	17.0%	15.9%
Buses	0	0	0	0	0	28	0	28	22	0	0	22	50
% Buses	0%	0%	0%	0%	0%	0.8%	0%	0.8%	0.7%	0%	0%	0.7%	0.7%

*L: Left, R: Right, T: Thru, U: U-Turn

SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

Full Length ()

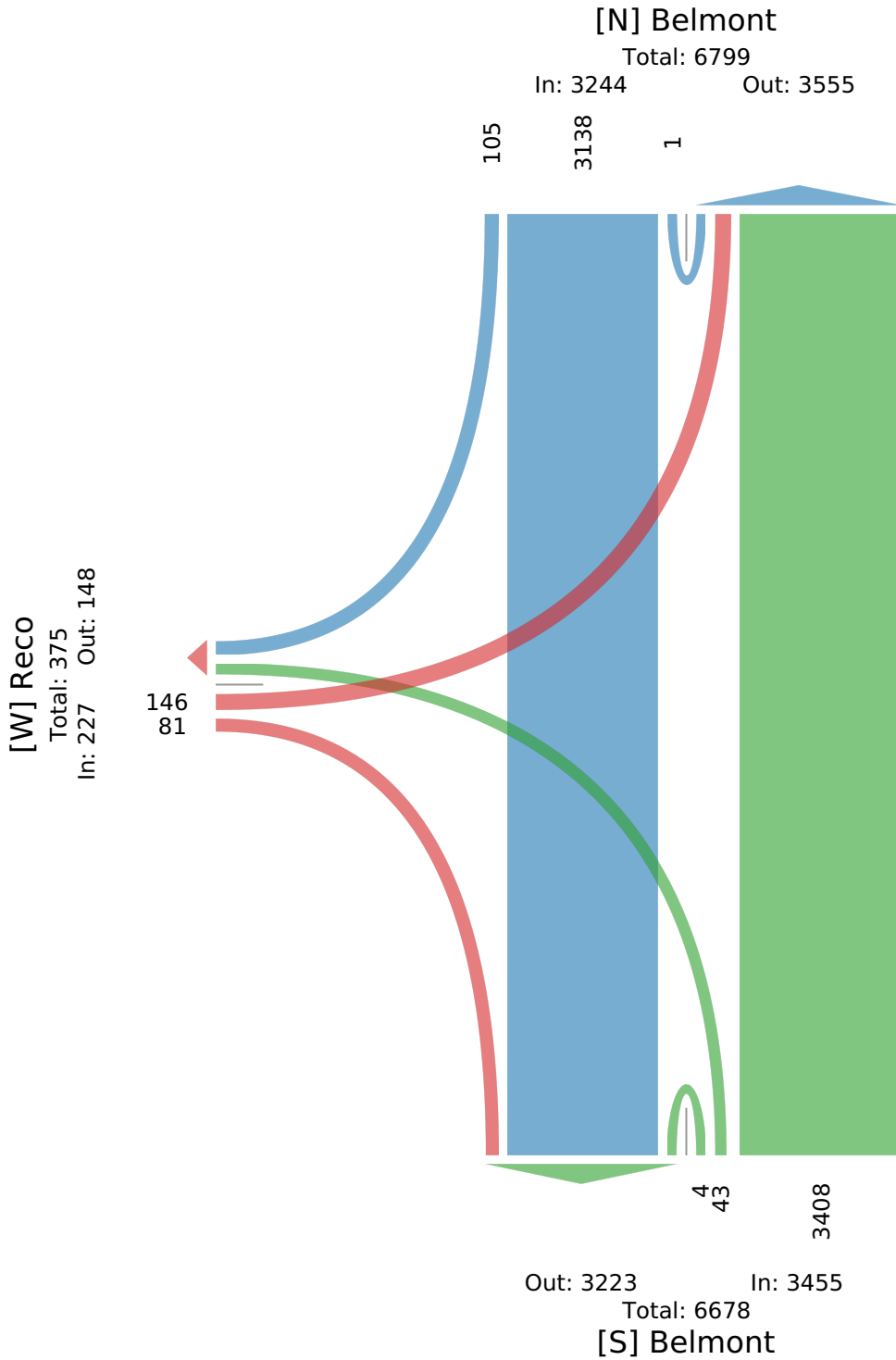
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.

625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	Reco Eastbound				Belmont Northbound				Belmont Southbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
Time													
2022-09-13 7:00AM	2	2	0	4	1	147	0	148	58	7	0	65	217
7:15AM	2	1	0	3	5	126	0	131	71	11	0	82	216
7:30AM	1	2	0	3	2	127	0	129	38	4	0	42	174
7:45AM	0	2	0	2	1	110	0	111	50	7	0	57	170
Total	5	7	0	12	9	510	0	519	217	29	0	246	777
% Approach	41.7%	58.3%	0%	-	1.7%	98.3%	0%	-	88.2%	11.8%	0%	-	-
% Total	0.6%	0.9%	0%	1.5%	1.2%	65.6%	0%	66.8%	27.9%	3.7%	0%	31.7%	-
PHF	0.625	0.875	-	0.750	0.450	0.867	-	0.877	0.764	0.659	-	0.750	0.895
Lights	4	7	0	11	5	469	0	474	178	27	0	205	690
% Lights	80.0%	100%	0%	91.7%	55.6%	92.0%	0%	91.3%	82.0%	93.1%	0%	83.3%	88.8%
Articulated Trucks and Single-Unit Trucks	1	0	0	1	4	36	0	40	35	2	0	37	78
% Articulated Trucks and Single-Unit Trucks	20.0%	0%	0%	8.3%	44.4%	7.1%	0%	7.7%	16.1%	6.9%	0%	15.0%	10.0%
Buses	0	0	0	0	0	5	0	5	4	0	0	4	9
% Buses	0%	0%	0%	0%	0%	1.0%	0%	1.0%	1.8%	0%	0%	1.6%	1.2%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

AM Peak (7 AM - 8 AM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

[N] Belmont

Total: 761

In: 246

Out: 515

29

217

[W] Reco

Total: 50

In: 12

Out: 38

5

7

9

510

Out: 224

In: 519

Total: 743

[S] Belmont

SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

Midday Peak (12 PM - 1 PM)

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	Reco Eastbound				Belmont Northbound				Belmont Southbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
2022-09-13 12:00PM	3	2	0	5	0	83	0	83	63	3	0	66	154
12:15PM	8	3	0	11	1	77	0	78	75	2	0	77	166
12:30PM	2	0	0	2	1	81	0	82	76	0	0	76	160
12:45PM	1	5	0	6	2	77	0	79	79	1	0	80	165
Total	14	10	0	24	4	318	0	322	293	6	0	299	645
% Approach	58.3%	41.7%	0%	-	1.2%	98.8%	0%	-	98.0%	2.0%	0%	-	-
% Total	2.2%	1.6%	0%	3.7%	0.6%	49.3%	0%	49.9%	45.4%	0.9%	0%	46.4%	-
PHF	0.438	0.500	-	0.545	0.500	0.958	-	0.970	0.927	0.500	-	0.934	0.971
Lights	10	9	0	19	3	261	0	264	226	5	0	231	514
% Lights	71.4%	90.0%	0%	79.2%	75.0%	82.1%	0%	82.0%	77.1%	83.3%	0%	77.3%	79.7%
Articulated Trucks and Single-Unit Trucks	4	1	0	5	1	57	0	58	66	1	0	67	130
% Articulated Trucks and Single-Unit Trucks	28.6%	10.0%	0%	20.8%	25.0%	17.9%	0%	18.0%	22.5%	16.7%	0%	22.4%	20.2%
Buses	0	0	0	0	0	0	0	0	1	0	0	1	1
% Buses	0%	0%	0%	0%	0%	0%	0%	0%	0.3%	0%	0%	0.3%	0.2%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

Midday Peak (12 PM - 1 PM)

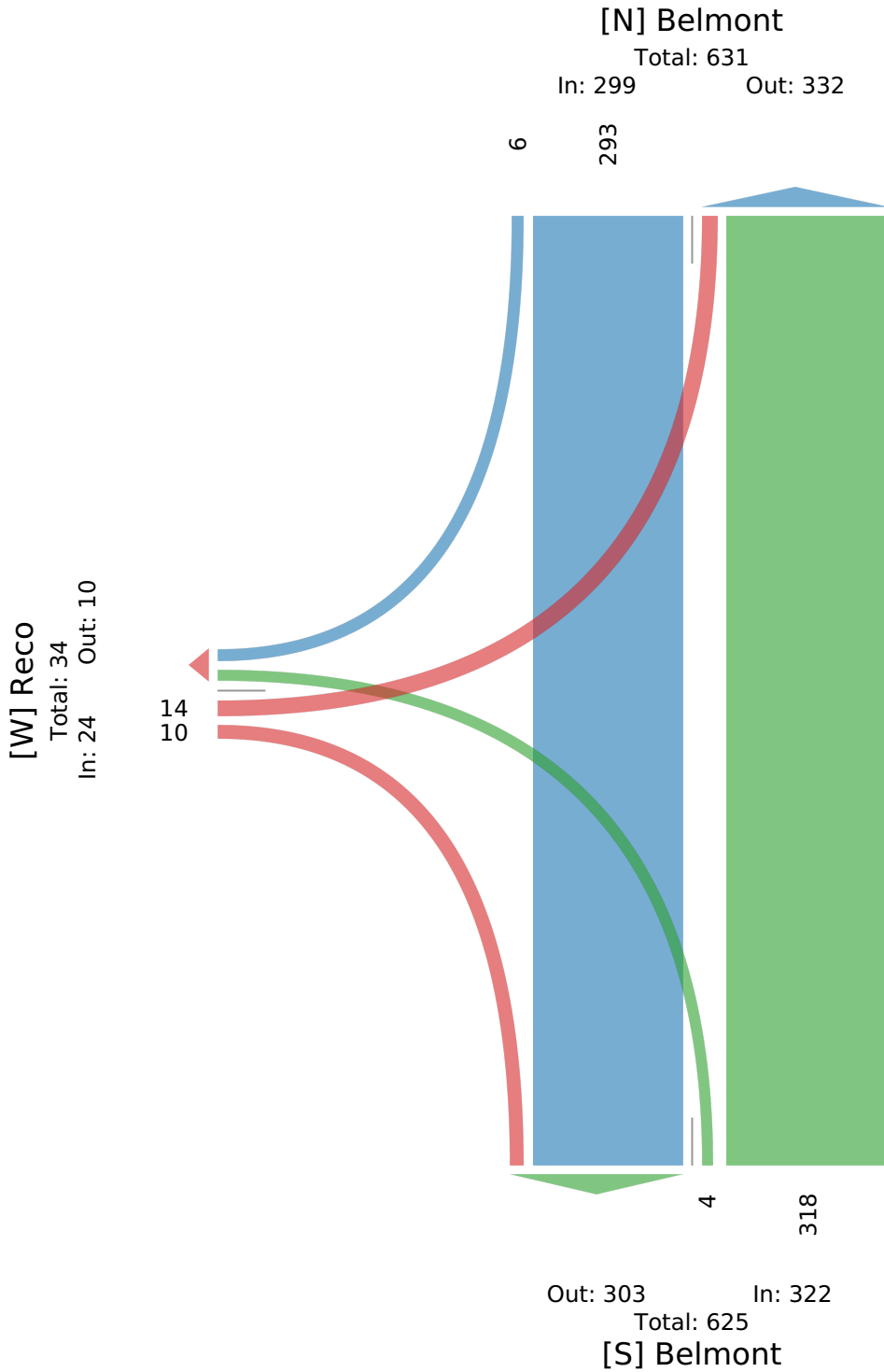
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US

Leg Direction	Reco Eastbound				Belmont Northbound				Belmont Southbound				Int
	L	R	U	App	L	T	U	App	T	R	U	App	
2022-09-13 4:30PM	3	3	0	6	1	100	3	104	109	0	0	109	219
4:45PM	5	1	0	6	1	105	0	106	102	2	0	104	216
5:00PM	11	2	0	13	0	106	0	106	128	3	0	131	250
5:15PM	0	2	0	2	2	102	0	104	137	2	0	139	245
Total	19	8	0	27	4	413	3	420	476	7	0	483	930
% Approach	70.4%	29.6%	0%	-	1.0%	98.3%	0.7%	-	98.6%	1.4%	0%	-	-
% Total	2.0%	0.9%	0%	2.9%	0.4%	44.4%	0.3%	45.2%	51.2%	0.8%	0%	51.9%	-
PHF	0.432	0.667	-	0.519	0.500	0.974	0.250	0.991	0.869	0.583	-	0.869	0.930
Lights	17	8	0	25	3	359	2	364	414	7	0	421	810
% Lights	89.5%	100%	0%	92.6%	75.0%	86.9%	66.7%	86.7%	87.0%	100%	0%	87.2%	87.1%
Articulated Trucks and Single-Unit Trucks	2	0	0	2	1	49	1	51	62	0	0	62	115
% Articulated Trucks and Single-Unit Trucks	10.5%	0%	0%	7.4%	25.0%	11.9%	33.3%	12.1%	13.0%	0%	0%	12.8%	12.4%
Buses	0	0	0	0	0	5	0	5	0	0	0	0	5
% Buses	0%	0%	0%	0%	0%	1.2%	0%	1.2%	0%	0%	0%	0%	0.5%

* L: Left, R: Right, T: Thru, U: U-Turn

SR-149 and Reco Merge 3 studies - TMC

Tue Sep 13, 2022

PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour

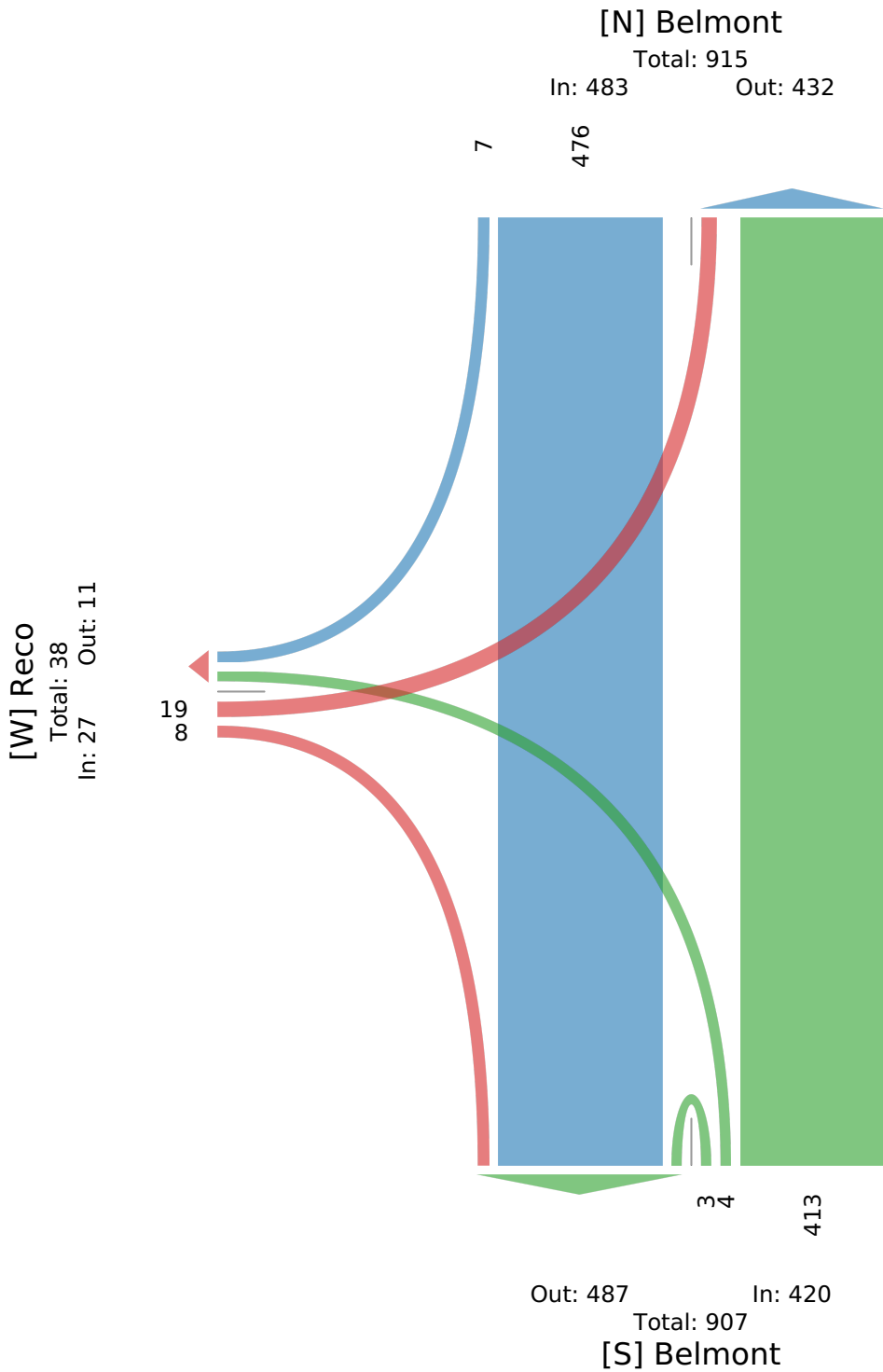
All Classes (Lights, Articulated Trucks and Single-Unit Trucks, Buses)

All Movements

ID: 1014819, Location: 40.058186, -81.051976



Provided by: Gewalt Hamilton Associates Inc.
625 Forest Edge Drive, Vernon Hills, IL, 60061, US



**APPENDIX C
GROWTH RATE AND DHV FACTOR
DOCUMENTATION**

Years	SR-149 N of I-70	SR-149 S of I-70	WB On Ramp	WB Off Ramp	EB On Ramp	EB Off Ramp
	-9.48%	-4.64%	-6.29%	-8.64%	-7.22%	-9.53%
1	0.9052	0.9536	0.9371	0.9136	0.9278	0.9047
21	-0.9908	0.0256	-0.3209	-0.8144	-0.5162	-1.0013

* In an effort to be conservative, CESO applied a growth rate of 0.5% to the study volumes.

PEAK HOUR to DESIGN HOUR FACTORS
FUNCTIONAL CLASSIFICATION = 01r
(Rural Interstate)

Day Month	Monthly Average by Day-of-Week							
	WEEKDAY MON- THUR	Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
January	1.65	1.68	1.75	1.72	1.62	1.52	1.34	1.75
February	1.59	1.58	1.69	1.63	1.57	1.49	1.20	1.66
March	1.48	1.33	1.54	1.54	1.47	1.36	1.13	1.50
April	1.42	1.27	1.50	1.48	1.40	1.31	1.11	1.46
May	1.37	1.27	1.44	1.41	1.37	1.26	1.06	1.35
June	1.32	1.18	1.38	1.37	1.30	1.22	1.05	1.29
July	1.29	1.13	1.36	1.33	1.27	1.21	1.04	1.22
August	1.33	1.15	1.40	1.37	1.32	1.23	1.05	1.24
September	1.41	1.25	1.49	1.45	1.40	1.29	1.05	1.32
October	1.38	1.18	1.47	1.42	1.36	1.26	1.04	1.33
November	1.44	1.35	1.49	1.45	1.45	1.36	1.10	1.46
December	1.47	1.57	1.57	1.53	1.42	1.37	1.19	1.54

peak hour volume * factor = design hour volume

source: year 2016, 2017, & 2018 Automatic Traffic Recorders (ATR) Data

ATR Stations:

2018: 135, 596, 608, 616, 617, 715, 745, 775, 787

2017: 135, 159, 596, 616, 617, 715, 745, 775

2016: 50, 135, 155, 159, 507, 596, 608, 610, 616, 617, 715, 775

Ohio Department of Transportatio
 Modeling & Forecasting Section
 June 2019

NOTE: These are NOT seasonal adjustment factors!!!

See Appendix A1 thru A4 for ATR sample statistics

PEAK HOUR to DESIGN HOUR FACTORS
FUNCTIONAL CLASSIFICATION = 05, 06r
(Rural Major Collector & Rural Minor Collector)

Day Month	Monthly Average by Day-of-Week							
	WEEKDAY MON- THUR	Sunday	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday
January	1.31	2.00	1.35	1.32	1.29	1.28	1.23	1.80
February	1.26	1.91	1.27	1.25	1.27	1.28	1.18	1.67
March	1.24	1.80	1.25	1.24	1.24	1.23	1.17	1.64
April	1.16	1.60	1.20	1.18	1.13	1.14	1.08	1.55
May	1.13	1.56	1.15	1.12	1.12	1.11	1.06	1.46
June	1.18	1.57	1.20	1.20	1.15	1.15	1.13	1.44
July	1.22	1.60	1.25	1.22	1.20	1.22	1.17	1.50
August	1.17	1.53	1.18	1.17	1.16	1.14	1.09	1.40
September	1.13	1.54	1.16	1.13	1.12	1.10	1.04	1.39
October	1.13	1.59	1.17	1.11	1.12	1.11	1.04	1.39
November	1.18	1.77	1.21	1.17	1.17	1.19	1.08	1.59
December	1.22	1.86	1.22	1.22	1.22	1.24	1.16	1.64

peak hour volume * factor = design hour volume

source: year 2016, 2017, & 2018 Automatic Traffic Recorders (ATR) Data

ATR Stations:

2018: 7, 67, 171, 516, 520, 548, 773, 774
 2017: 7, 67, 171, 516, 520, 773, 774, 548, 549
 2016: 7, 67, 516, 520, 773, 774, 548, 549

Ohio Department of Transportatio
 Modeling & Forecasting Section
 June 2019

NOTE: These are NOT seasonal adjustment factors!!!

Note: Insufficient data exists to produce factors for functional class 07 Rural.

**APPENDIX D
ITE TRIP GENERATION RESOURCES
AND CALCULATIONS**

Land Use: 945

Convenience Store/Gas Station

Description

A convenience store/gas station is a facility with a co-located convenience store and gas station. The convenience store sells grocery and other everyday items that a person may need or want as a matter of convenience. The gas station sells automotive fuels such as gasoline and diesel.

A convenience store/gas station is typically located along a major thoroughfare to optimize motorist convenience. Extended hours of operation (with many open 24 hours, 7 days a week) are common at these facilities.

The convenience store product mix typically includes pre-packaged grocery items, beverages, dairy products, snack foods, confectionary, tobacco products, over-the-counter drugs, and toiletries. A convenience store may sell alcohol, often limited to beer and wine. Coffee and pre-made sandwiches are also commonly sold at a convenience store. Made-to-order food orders are sometimes offered. Some stores offer limited seating.

The sites in this land use include both self-pump and attendant-pumped fueling positions and both pre-pay and post-pay operations.

Convenience store (Land Use 851), gasoline/service station (Land Use 944), and truck stop (Land Use 950) are related uses.

Land Use Subcategory

Multiple subcategories were added to this land use to allow for multi-variable evaluation of sites with single-variable data plots. All study sites are assigned to one of three subcategories, based on the number of vehicle fueling positions (VFP) at the site: between 2 and 8 VFP, between 9 and 15 VFP, and between 16 and 24 VFP. For each VFP range subcategory, data plots are presented with GFA as the independent variable for all time periods and trip types for which data are available. The use of both GFA and VFP (as the independent variable and land use subcategory, respectively) provides a significant improvement in the reliability of a trip generation estimate when compared to the single-variable data plots in prior editions of *Trip Generation Manual*.

Further, the study sites were also assigned to one of three other subcategories, based on the gross floor area (GFA) of the convenience store at the site: between 2,000 and 4,000 square feet, between 4,000 and 5,500 square feet, and between 5,500 and 10,000 square feet. For each GFA subcategory range, data plots are presented with VFP as the independent variable for all time periods and trip types for which data are available. The use of both VFP and GFA (as the independent variable and land use subcategory, respectively) provides a significant improvement in the reliability of a trip generation estimate when compared to the single-variable data plots in prior editions of *Trip Generation Manual*.

When analyzing the convenience store/gas station land use with each combination of GFA and VFP values as described above, the two sets of data plots will produce two estimates of site-generated trips. Both values can be considered when determining a site trip generation estimate.

Data plots are also provided for three additional independent variables: AM peak hour traffic on adjacent street, PM peak hour traffic on adjacent street, and employees. These independent variables are intended to be analyzed as single independent variables and do not have sub-categories associated with them. Within the data plots and within the ITETripGen web app, these plots are found under the land use subcategory “none.”

Additional Data

ITE recognizes there are existing convenience store/gas station sites throughout North America that are larger than the sites presented in the data plots. However, the ITE database does not include any site with more than 24 VFP or any site with gross floor area greater than 10,000 square feet. Submission of trip generation data for larger sites is encouraged.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (<https://www.ite.org/technical-resources/topics/trip-and-parking-generation/>).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), Arkansas, California, Connecticut, Delaware, Florida, Indiana, Iowa, Kentucky, Maryland, Massachusetts, Minnesota, Nevada, New Hampshire, New Jersey, Pennsylvania, Rhode Island, South Dakota, Texas, Utah, Vermont, Washington, and Wisconsin.

Source Numbers

221, 245, 274, 288, 300, 340, 350, 351, 352, 355, 359, 385, 440, 617, 718, 810, 813, 844, 850, 853, 864, 865, 867, 869, 882, 883, 888, 904, 926, 927, 936, 938, 954, 960, 962, 977, 1004, 1024, 1025, 1027, 1052

Convenience Store/Gas Station - GFA (4-5.5k) (945)

Vehicle Trip Ends vs: Vehicle Fueling Positions
On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 5

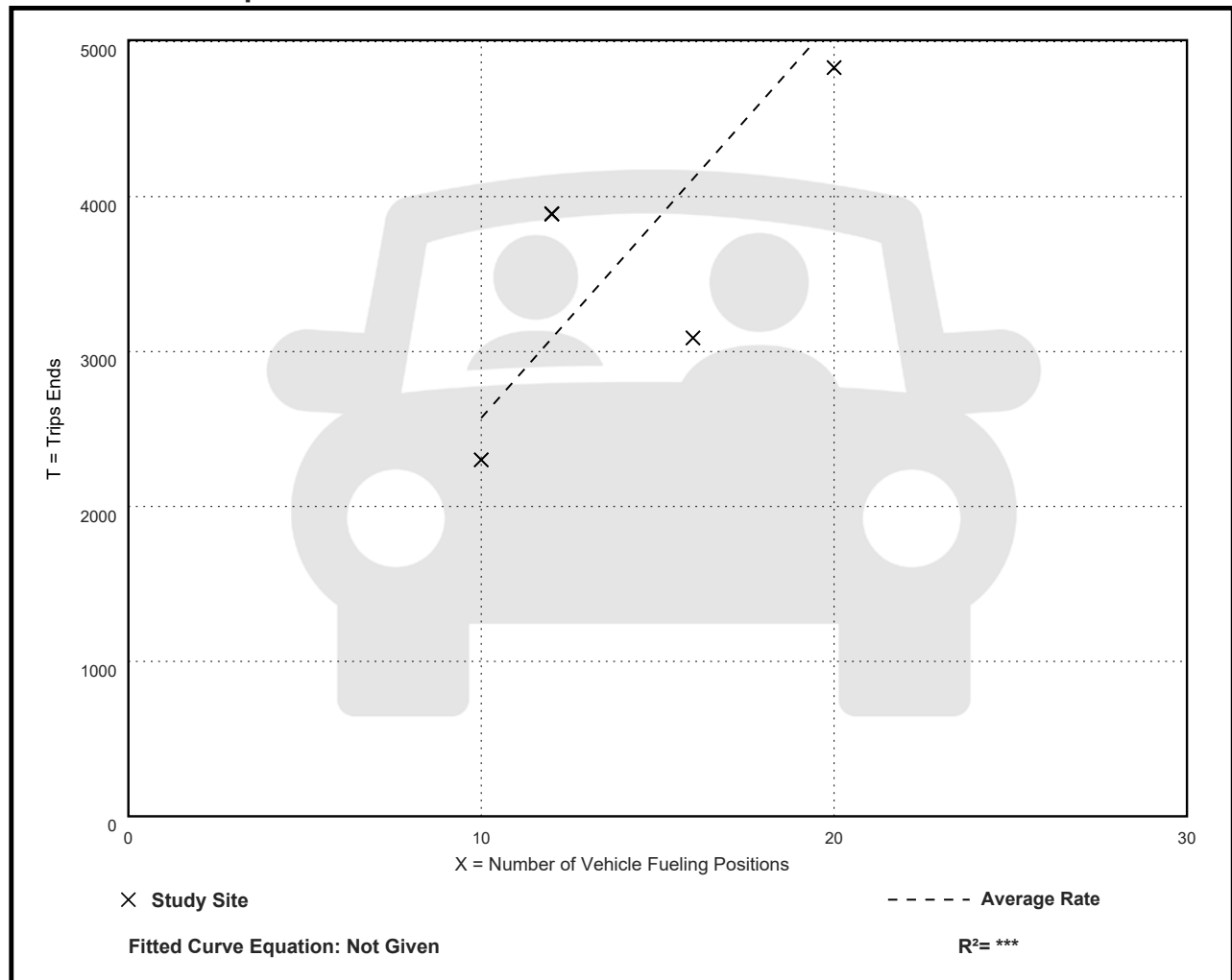
Avg. Num. of Vehicle Fueling Positions: 14

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
257.13	193.00 - 324.17	57.53

Data Plot and Equation



Convenience Store/Gas Station - GFA (4-5.5k) (945)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 18

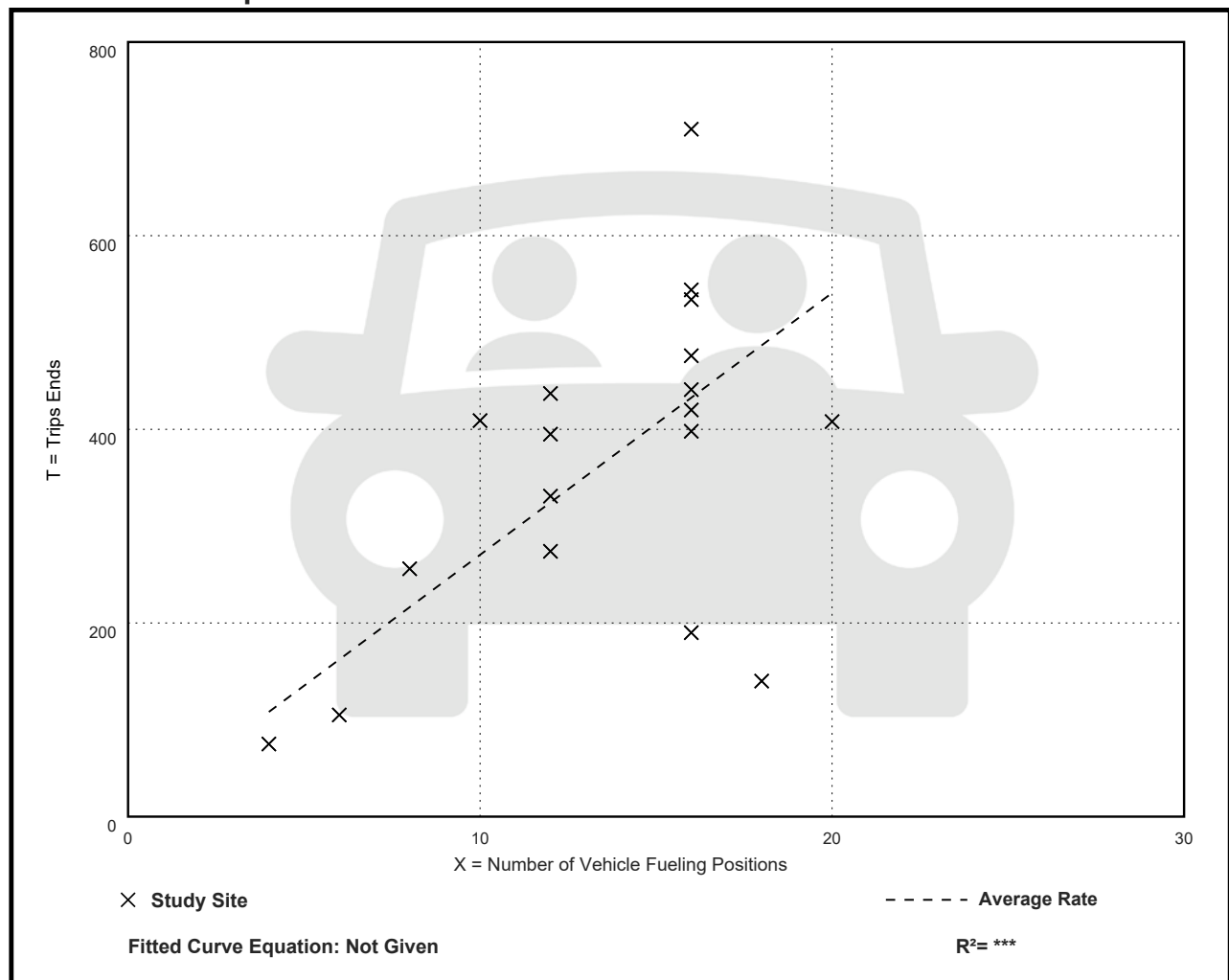
Avg. Num. of Vehicle Fueling Positions: 13

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
27.04	7.78 - 44.38	9.88

Data Plot and Equation



Convenience Store/Gas Station - GFA (4-5.5k) (945)

Vehicle Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 23

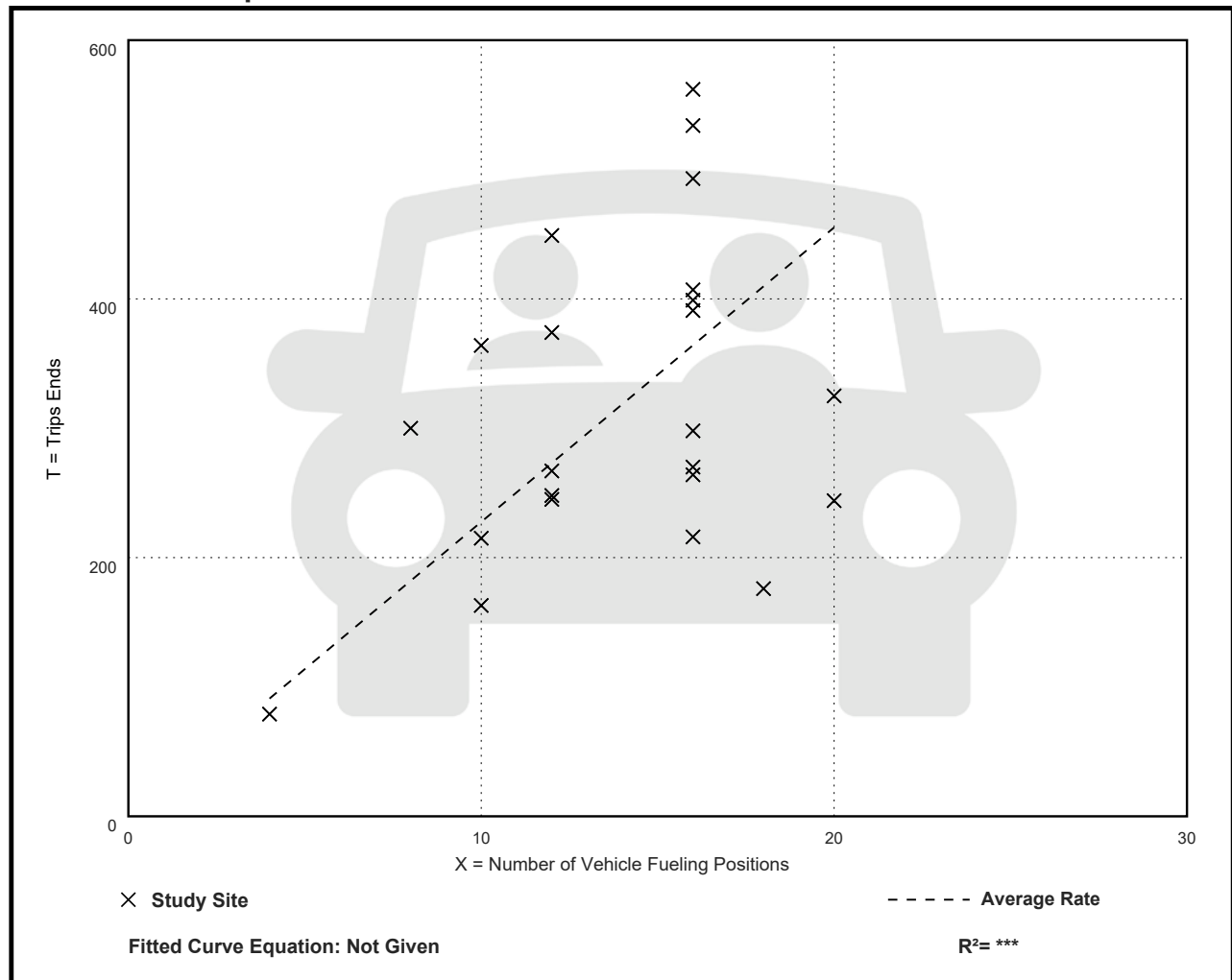
Avg. Num. of Vehicle Fueling Positions: 14

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
22.76	9.78 - 37.50	8.49

Data Plot and Equation



September 30, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 945 – Gasoline/Service Station with Convenience Market

For Weekday → 50% Enter/50% Exit

257.13 x 16 Passenger Car Fueling Positions = 4,114.08 ≈ 4,114 Trips for Even Number

4,114 Trips x 0.50 (50%) = 2,057 Trips Enter/2,057 Trips Exit

For AM Peak Hour → 50% Enter/50% Exit

27.04 x 16 Passenger Car Fueling Positions = 432.64 ≈ 432 Trips for Even Number

Pass-by Trips = 432 Trips x 0.76 (76%) = 328 Trips

Pass-by Trips = 328 Trips x 0.50 (50%) = 164 Trips Enter/164 Trips Exit

Primary Trips = 432 – 328 = 104 Trips

Primary Trips = 104 x 0.50 = 52 Trips Enter/52 Trips Exit

For PM Peak Hour → 50% Enter/50% Exit

22.76 x 16 Passenger Car Fueling Positions = 364.16 ≈ 365 Trips for Even Number

Pass-by Trips = 365 Trips x 0.75 (75%) = 274 Trips

Pass-by Trips = 274 Trips x 0.50 (50%) = 137 Trips Enter/137 Trips Exit

Primary Trips = 365 – 274 = 91 Trips

Primary Trips = 91 x 0.50 (50%) = 46 Trips Enter/45 Trips Exit

Land Use: 934

Fast-Food Restaurant with Drive-Through Window

Description

This land use includes any fast-food restaurant with a drive-through window. This type of restaurant is characterized by a large drive-through and large carry-out clientele, long hours of service (some are open for breakfast, all are open for lunch and dinner, some are open late at night or 24 hours a day) and high turnover rates for eat-in customers. The restaurant does not provide table service. A patron generally orders from a menu board and pays before receiving the meal. A typical duration of stay for an eat-in patron is less than 30 minutes. Fast casual restaurant (Land Use 930), high-turnover (sit-down) restaurant (Land Use 932), fast-food restaurant without drive-through window (Land Use 933), and fast-food restaurant with drive-through window and no indoor seating (Land Use 935) are related uses.

Additional Data

Users should exercise caution when applying statistics during the AM peak periods, as the sites contained in the database for this land use may or may not be open for breakfast. In cases where it was confirmed that the sites were not open for breakfast, data for the AM peak hour of the adjacent street traffic were removed from the database.

If the restaurant has outdoor seating, its area is not included in the overall gross floor area. For a restaurant that has significant outdoor seating, the number of seats may be more reliable than GFA as an independent variable on which to establish a trip generation rate.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (<https://www.ite.org/technical-resources/topics/trip-and-parking-generation/>).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alaska, Alberta (CAN), California, Colorado, Florida, Indiana, Kentucky, Maryland, Massachusetts, Minnesota, Montana, New Hampshire, New Jersey, New York, North Carolina, Ohio, Pennsylvania, South Dakota, Texas, Vermont, Virginia, Washington, and Wisconsin.

Source Numbers

163, 164, 168, 180, 181, 241, 245, 278, 294, 300, 301, 319, 338, 340, 342, 358, 389, 438, 502, 552, 577, 583, 584, 617, 640, 641, 704, 715, 728, 810, 866, 867, 869, 885, 886, 927, 935, 962, 977, 1050, 1053, 1054

Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
 On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 71

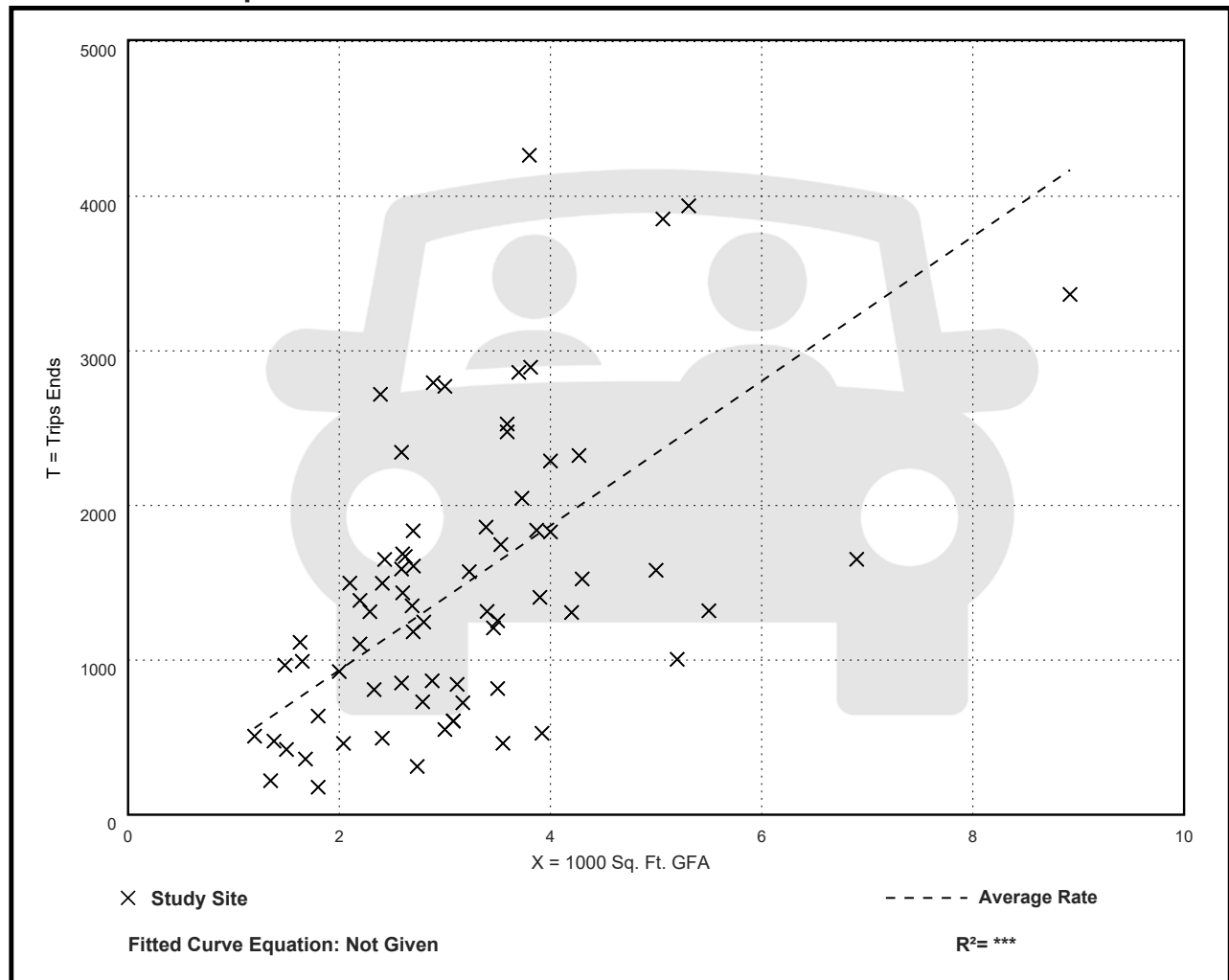
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
467.48	98.89 - 1137.66	238.62

Data Plot and Equation



Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 96

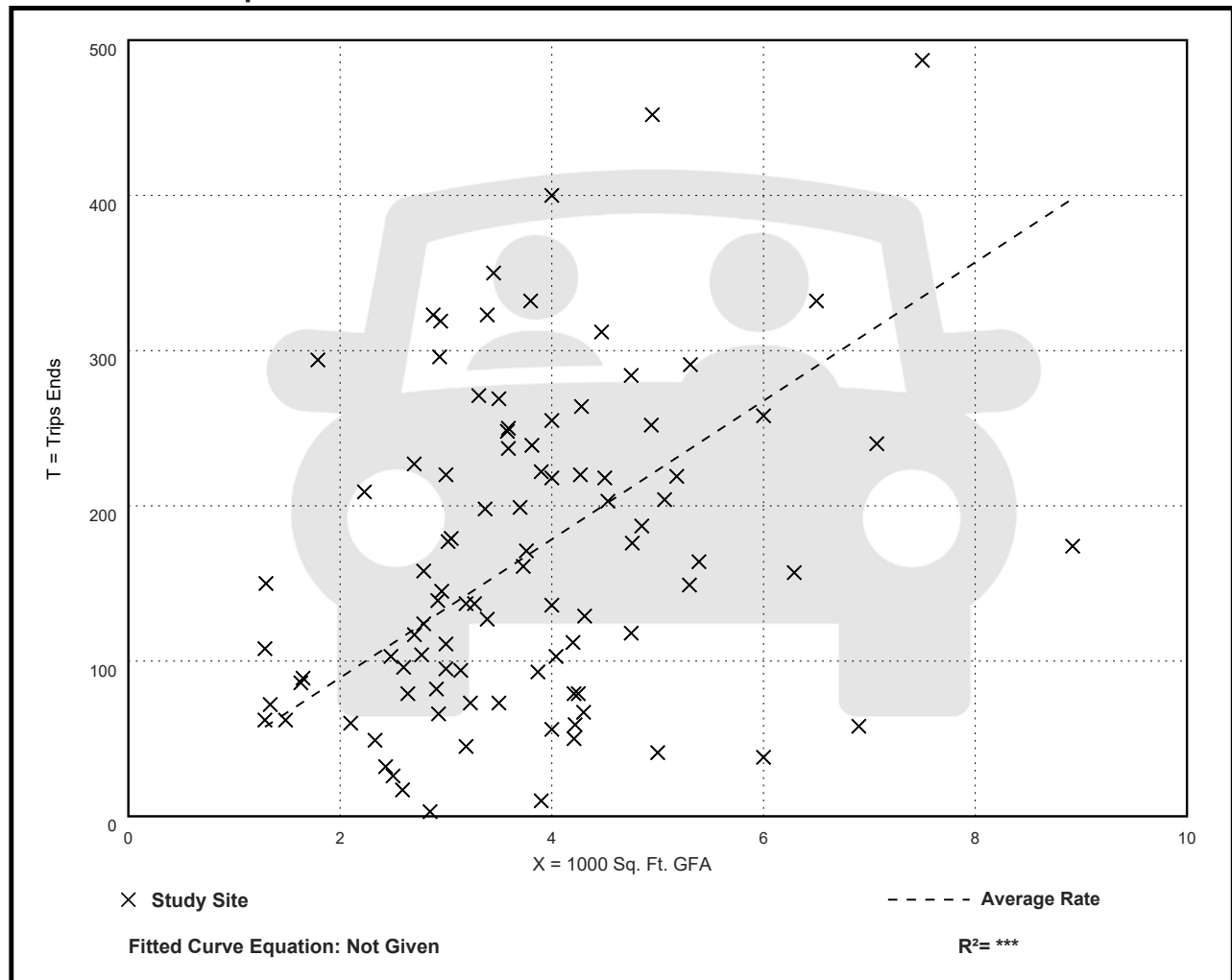
Avg. 1000 Sq. Ft. GFA: 4

Directional Distribution: 51% entering, 49% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
44.61	1.05 - 164.25	27.14

Data Plot and Equation



Fast-Food Restaurant with Drive-Through Window (934)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 190

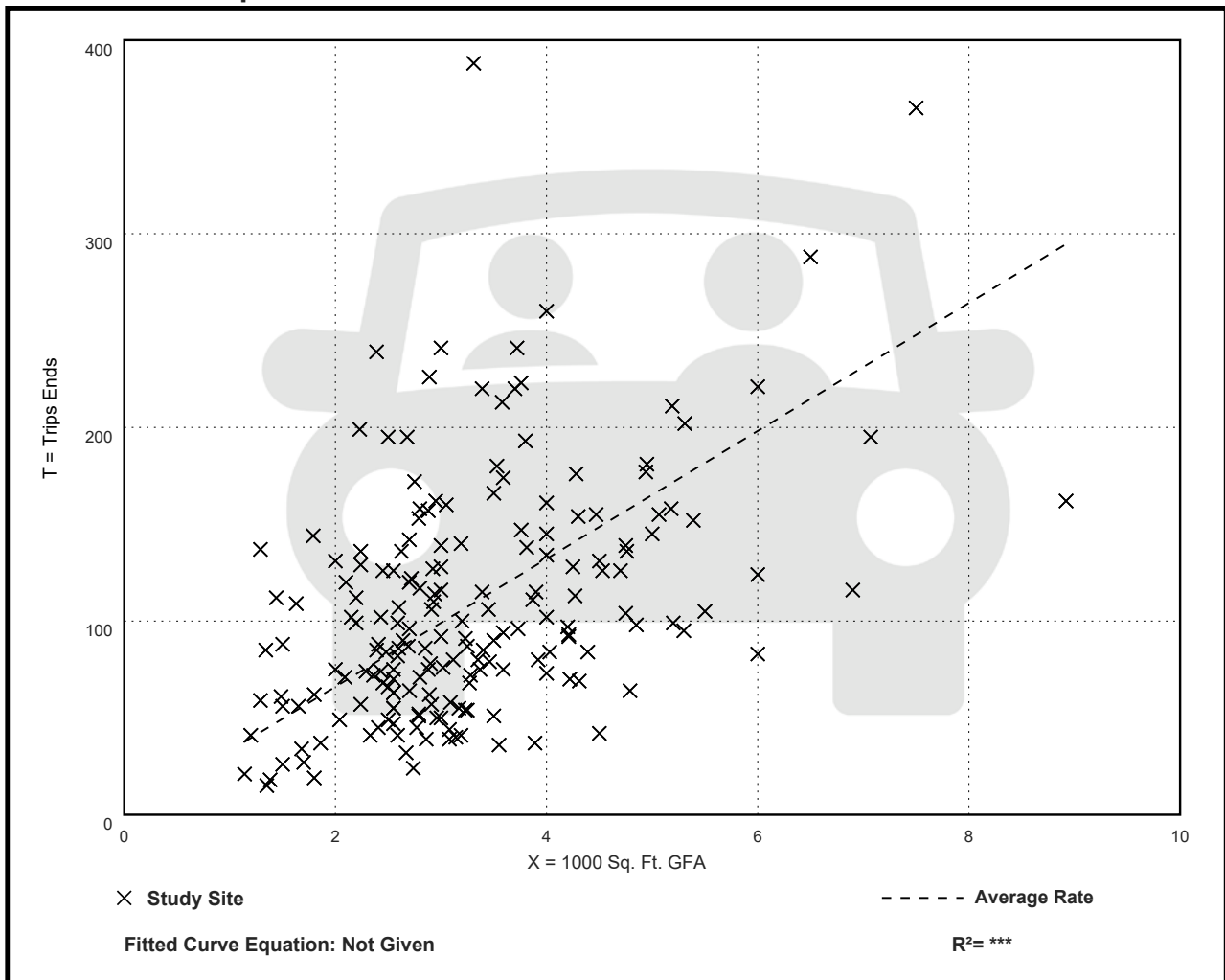
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 52% entering, 48% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
33.03	8.77 - 117.22	17.59

Data Plot and Equation



September 30, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 934 – Fast-Food Restaurant with Drive-Through Window

For Weekday → 50% Enter/50% Exit

$3.10 \text{ KSF} \times 467.48 = 1,449.18 \approx 1,450 \text{ Trips}$ for Even Number

$1,450 \text{ Trips} \times 0.50 (50\%) = 725 \text{ Trips Enter}/725 \text{ Trips Exit}$

For AM Peak Hour → 51% Enter/49% Exit

$3.10 \text{ KSF} \times 44.61 = 138.29 \approx 138 \text{ Trips}$

Pass-by Trips = $138 \text{ Trips} \times 0.50 (50\%) = 70 \text{ Trips}$ for Even Number

Pass-by Trips = $70 \text{ Trips} \times 0.50 (50\%) = 35 \text{ Trips Enter}/35 \text{ Trips Exit}$

Primary Trips = $138 - 70 = 68 \text{ Trips}$

Primary Trips = $68 \times 0.51 (51\%)$ and $68 \times 0.49 (49\%) = 35 \text{ Trips Enter}/33 \text{ Trips Exit}$

For PM Peak Hour → 52% Enter/48% Exit

$3.10 \text{ KSF} \times 33.03 = 102.39 \approx 102 \text{ Trips}$

Pass-by Trips = $102 \text{ Trips} \times 0.55 (55\%) = 56 \text{ Trips}$ for Even Number

Pass-by Trips = $56 \text{ Trips} \times 0.50 (50\%) = 28 \text{ Trips Enter}/28 \text{ Trips Exit}$

Primary Trips = $102 - 56 = 46 \text{ Trips}$

Primary Trips = $46 \times 0.52 (52\%)$ and $46 \times 0.48 (48\%) = 24 \text{ Trips Enter}/22 \text{ Trips Exit}$

Land Use: 933

Fast-Food Restaurant without Drive-Through Window

Description

This land use includes any fast-food restaurant without a drive-through window. This type of restaurant is characterized by a large carry-out clientele, long hours of service (some are open for breakfast, all are open for lunch and dinner, some are open late at night or 24 hours a day) and high turnover rates for eat-in customers. These limited-service eating establishments do not provide table service. A patron generally orders from a menu board and pays before receiving the meal. A typical duration of stay for an eat-in customer is less than 30 minutes. Fast casual restaurant (Land Use 930), high-turnover (sit-down) restaurant (Land Use 932), and fast-food restaurant with drive-through window (Land Use 934) are related uses.

Additional Data

If the restaurant has outdoor seating, its area is not included in the overall gross floor area. For a restaurant that has significant outdoor seating, the number of seats may be more reliable than GFA as an independent variable on which to establish a trip generation rate.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (<https://www.ite.org/technical-resources/topics/trip-and-parking-generation/>).

The sites were surveyed in the 1980s and the 2010s in Alberta (CAN), California, Colorado, Connecticut, Maryland, Montana, Pennsylvania, and Texas.

Source Numbers

163, 247, 278, 319, 342, 885, 977, 1020

Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 6

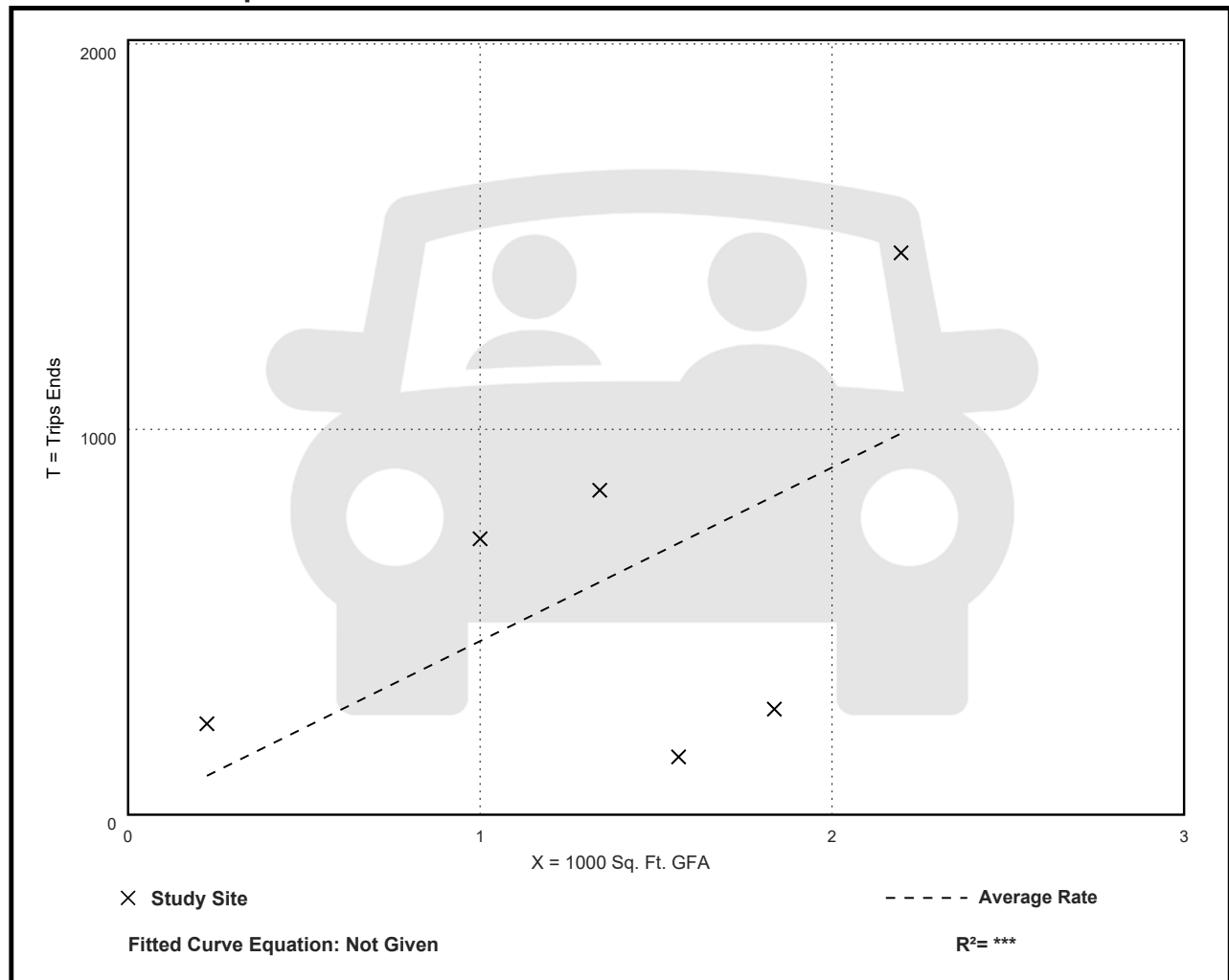
Avg. 1000 Sq. Ft. GFA: 1

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
450.49	95.91 - 1053.57	310.99

Data Plot and Equation



Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 3

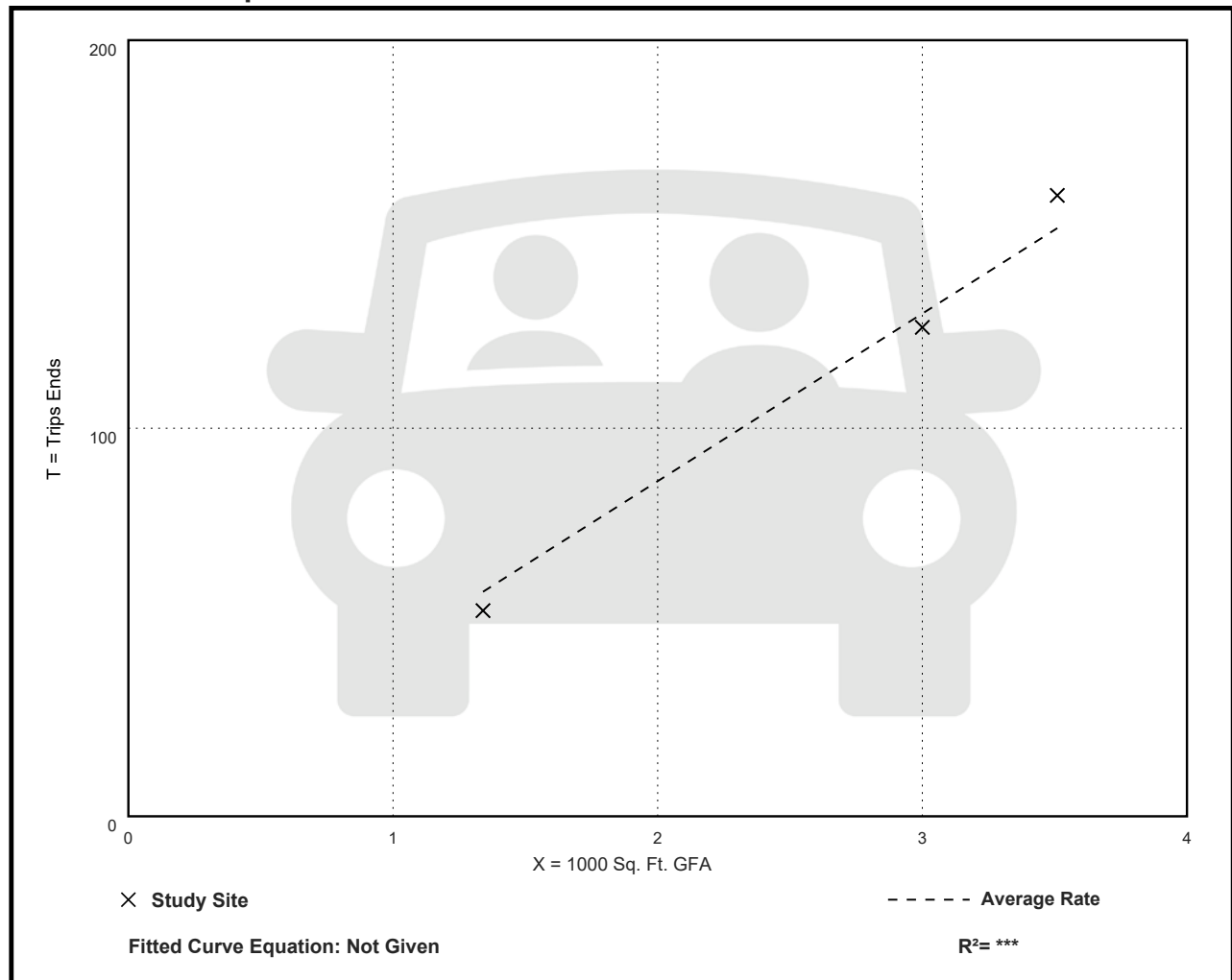
Avg. 1000 Sq. Ft. GFA: 3

Directional Distribution: 58% entering, 42% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
43.18	39.55 - 45.58	2.84

Data Plot and Equation



Fast-Food Restaurant without Drive-Through Window (933)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 8

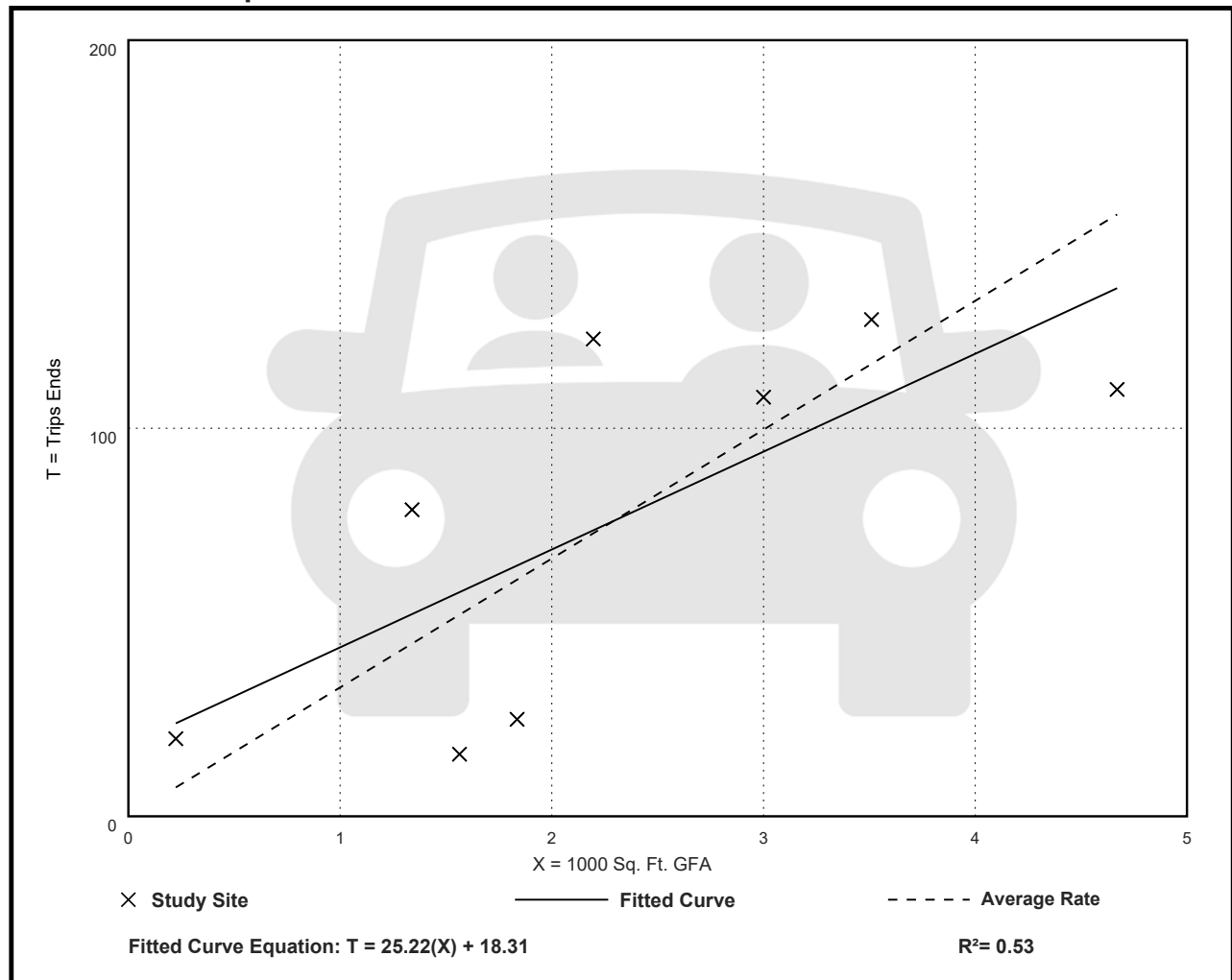
Avg. 1000 Sq. Ft. GFA: 2

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
33.21	10.23 - 89.29	17.22

Data Plot and Equation



September 30, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 933 – Fast-Food Restaurant without Drive-Through Window

For Weekday → 50% Enter/50% Exit

$2.90 \text{ KSF} \times 450.49 = 1,306.42 \approx 1,306 \text{ Trips}$ for Even Number

$1,306 \text{ Trips} \times 0.50 (50\%) = 653 \text{ Trips Enter}/653 \text{ Trips Exit}$

For AM Peak Hour → 58% Enter/42% Exit

$2.90 \text{ KSF} \times 43.18 = 125.22 \approx 125 \text{ Trips}$

Pass-by Trips = $125 \text{ Trips} \times 0.00 (0\%) = 0 \text{ Trips}$ for Even Number

Primary Trips = $125 - 0 = 125 \text{ Trips}$

Primary Trips = $125 \times 0.58 (58\%)$ and $125 \times 0.42 (42\%) = 73 \text{ Trips Enter}/52 \text{ Trips Exit}$

For PM Peak Hour → 50% Enter/50% Exit

$2.90 \text{ KSF} \times 33.21 = 96.31 \approx 96 \text{ Trips}$

Pass-by Trips = $96 \text{ Trips} \times 0.00 (0\%) = 96 \text{ Trips}$ for Even Number

Primary Trips = $96 - 0 = 96 \text{ Trips}$

Primary Trips = $96 \times 0.50 (50\%) = 48 \text{ Trips Enter}/48 \text{ Trips Exit}$

Land Use: 416

Campground/Recreational Vehicle Park

Description

A campground/recreational vehicle park is a recreational site that accommodates campers, trailers, tents, and recreational vehicles on a transient basis. They are found in a variety of locations and provide a variety of facilities, often including restrooms with showers and recreational facilities, such as a swimming pool, convenience store, and laundromat.

Additional Data

The sites were surveyed in the 1990s, the 2000s, and the 2010s in Rhode Island, Vermont, and Washington.

Source Numbers

401, 559, 728

Campground/Recreational Vehicle Park (416)

Vehicle Trip Ends vs: Occupied Campsites

On a: **Weekday,**

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 4

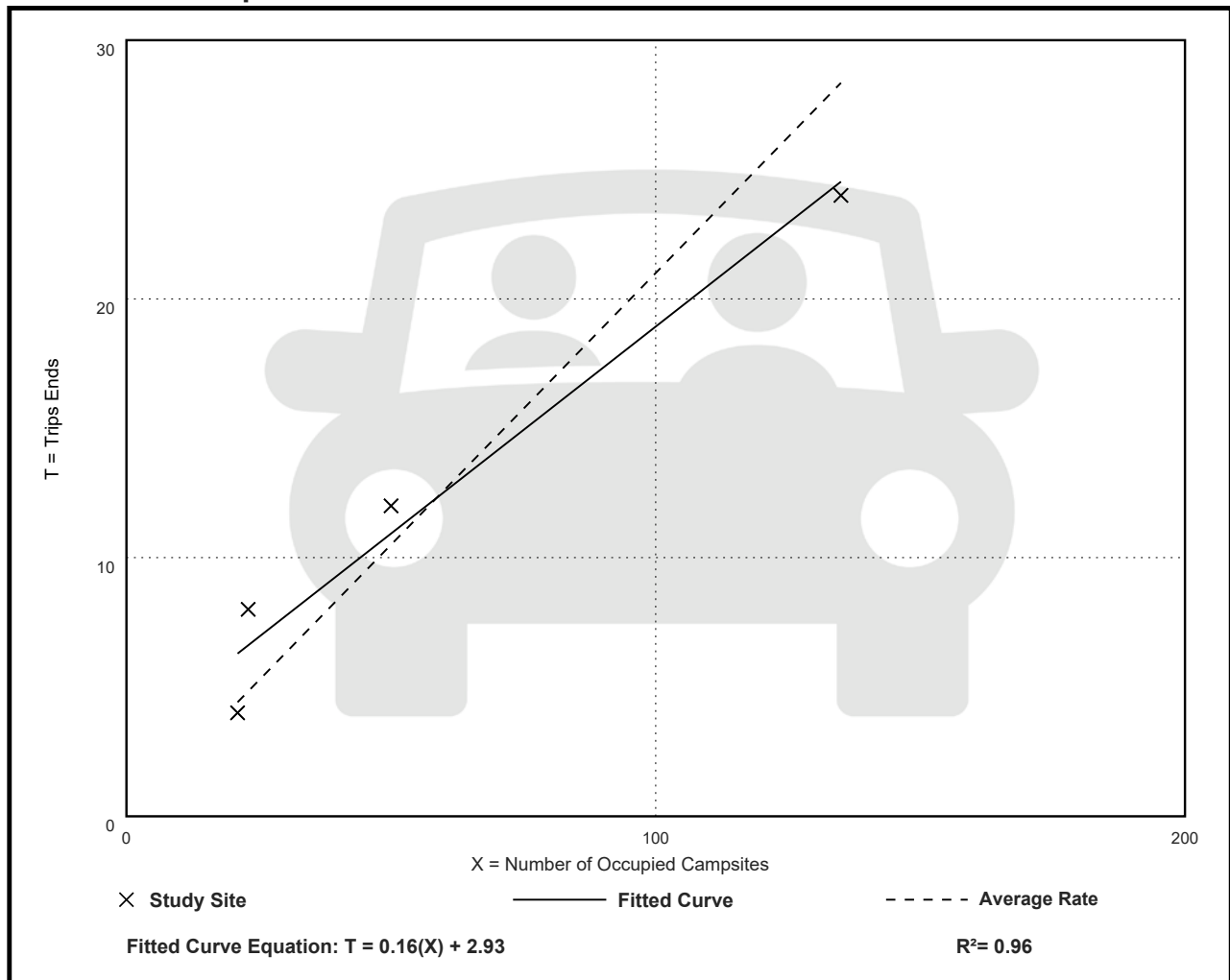
Avg. Num. of Occupied Campsites: 57

Directional Distribution: 36% entering, 64% exiting

Vehicle Trip Generation per Occupied Campsite

Average Rate	Range of Rates	Standard Deviation
0.21	0.18 - 0.35	0.06

Data Plot and Equation



Campground/Recreational Vehicle Park (416)

Vehicle Trip Ends vs: Occupied Campsites

On a: **Weekday,**

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 6

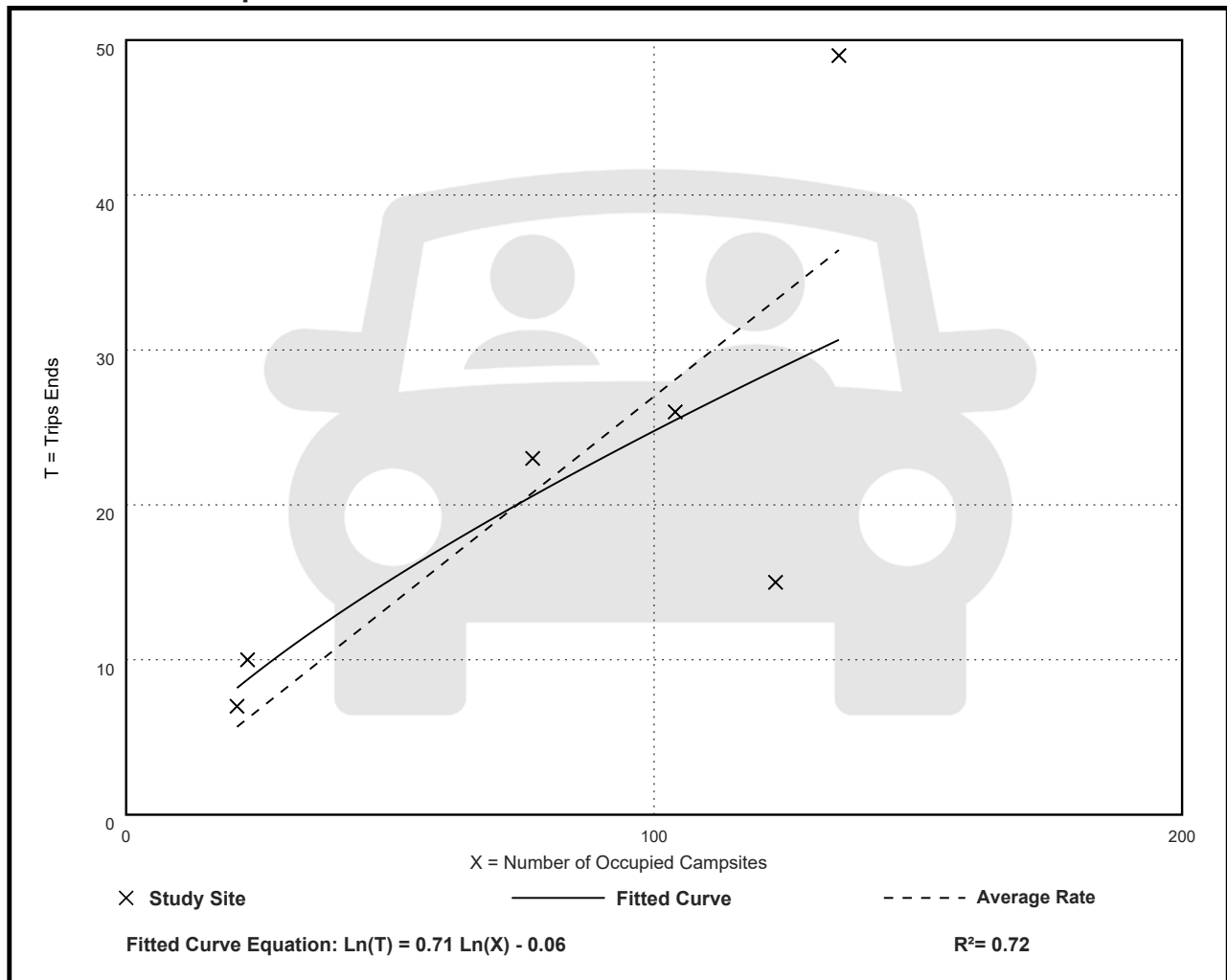
Avg. Num. of Occupied Campsites: 81

Directional Distribution: 65% entering, 35% exiting

Vehicle Trip Generation per Occupied Campsite

Average Rate	Range of Rates	Standard Deviation
0.27	0.12 - 0.43	0.11

Data Plot and Equation



November 15, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 416 – Campground/Recreational Vehicle Park

For Weekday → N/A

For AM Peak Hour → 36% Enter/64% Exit

70 Occupied Campsites x 0.21 = 14.7 ≈ **15 Trips**

Pass-by Trips = 15 Trips x 0.00 (0%) = **0 Trips** for Even Number

Primary Trips = 15 – 0 = 15 Trips

Primary Trips = 15 x 0.36 (36%) and 15 x 0.64 (64%) = **5 Trips Enter/10 Trips Exit**

For PM Peak Hour → 65% Enter/35% Exit

70 Occupied Campsites x 0.27 = 18.9 ≈ **19 Trips**

Pass-by Trips = 19 Trips x 0.00 (0%) = **0 Trips** for Even Number

Primary Trips = 19 – 0 = 19 Trips

Primary Trips = 19 x 0.65 (65%) and 19 x 0.35 (35%) = **12 Trips Enter/7 Trips Exit**

Land Use: 950 Truck Stop

Description

A truck stop is a facility located adjacent to an interstate highway interchange that provides commercial vehicle fueling, space and supplies for self-service vehicle maintenance, and other services specific to the needs of truckers (e.g., showers, on-site truck parking area). The facility typically contains a convenience store, restroom facilities, and one or more restaurants (either fast-food or high-turnover sit-down). Gasoline/service station (Land Use 944) and convenience store/gas station (Land Use 945) are related uses.

Additional Data

The trip generation data presented for this land use constitute commercial truck trips only.

The independent variable used in the data plots (vehicle fueling positions) refers to only the commercial fueling lanes at the truck stop.

The convenience store, restrooms, and restaurant(s) associated with a truck stop typically are also open to the general motoring public. The site often also includes vehicle fueling positions for the general motoring public. Additional information is needed in order to estimate non-truck vehicle trips generated by truck stops.

The truck trip generation rates per vehicle fueling position appear unreasonably high considering the length of time a trucker needs to fuel a vehicle. However, the detailed information provided in the data sources support the validity of the truck counts. It is apparent that not all trucks entering or exiting the truck stop are refueled. In those circumstances, the trucker could be using other services provided by the truck stop.

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (<https://www.ite.org/technical-resources/topics/trip-and-parking-generation/>).

The sites were surveyed in the 2000s and the 2010s in Colorado, Florida, Tennessee, Vermont, and Virginia.

To assist in the future analysis of this land use, it is important that the number of gasoline and diesel pumps at the study site be reported. It is also important to collect additional information on the number of non-truck trips generated at these sites.

Source Numbers

721, 913, 920, 927

Truck Stop (950)

Truck Trip Ends vs: Vehicle Fueling Positions
On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 2

Avg. Num. of Vehicle Fueling Positions: 9

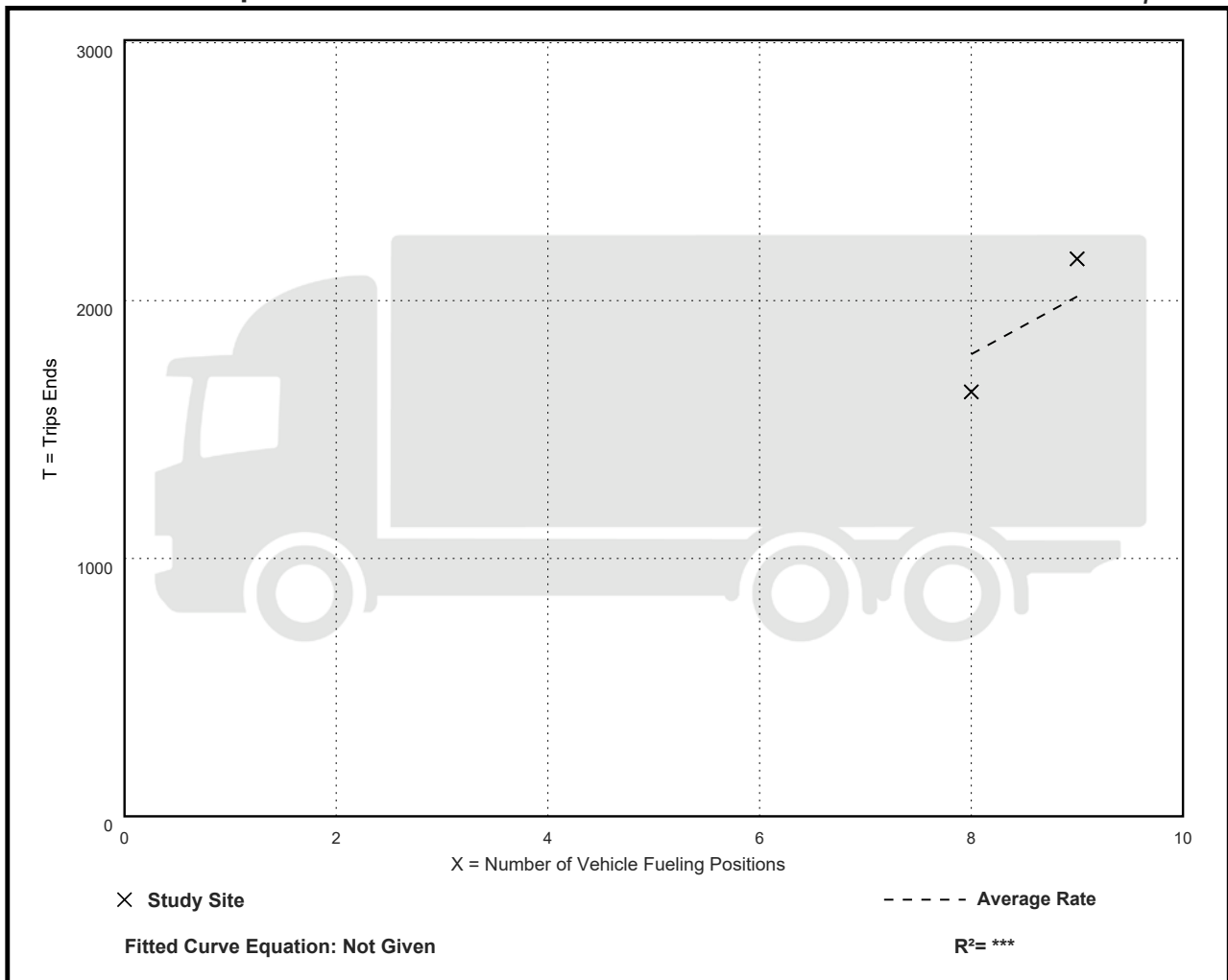
Directional Distribution: 50% entering, 50% exiting

Truck Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
224.00	205.75 - 240.22	***

Data Plot and Equation

Caution – Small Sample Size



Truck Stop (950)

Truck Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 4

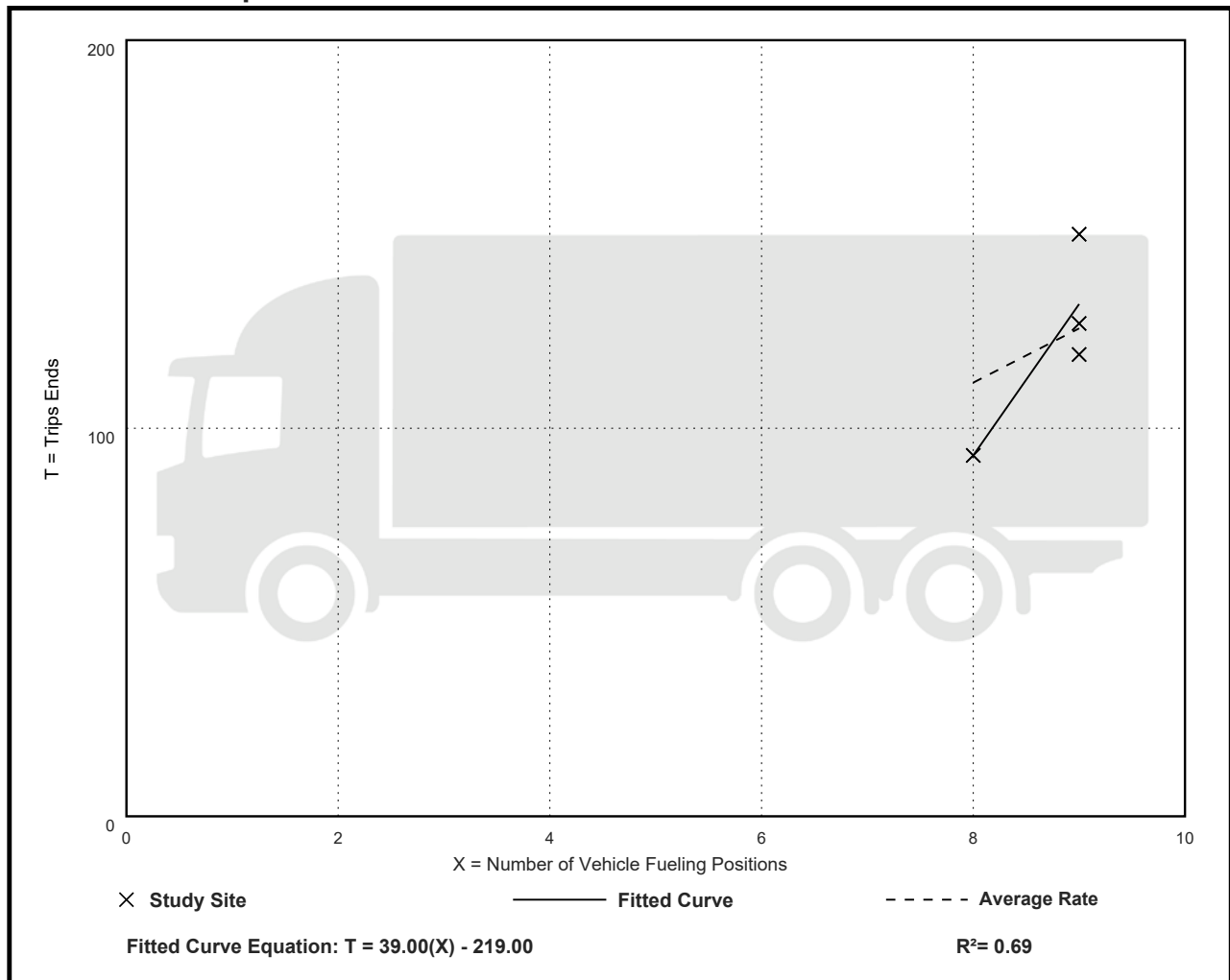
Avg. Num. of Vehicle Fueling Positions: 9

Directional Distribution: 49% entering, 51% exiting

Truck Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
13.97	11.63 - 16.67	2.09

Data Plot and Equation



Truck Stop (950)

Truck Trip Ends vs: Vehicle Fueling Positions

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 7

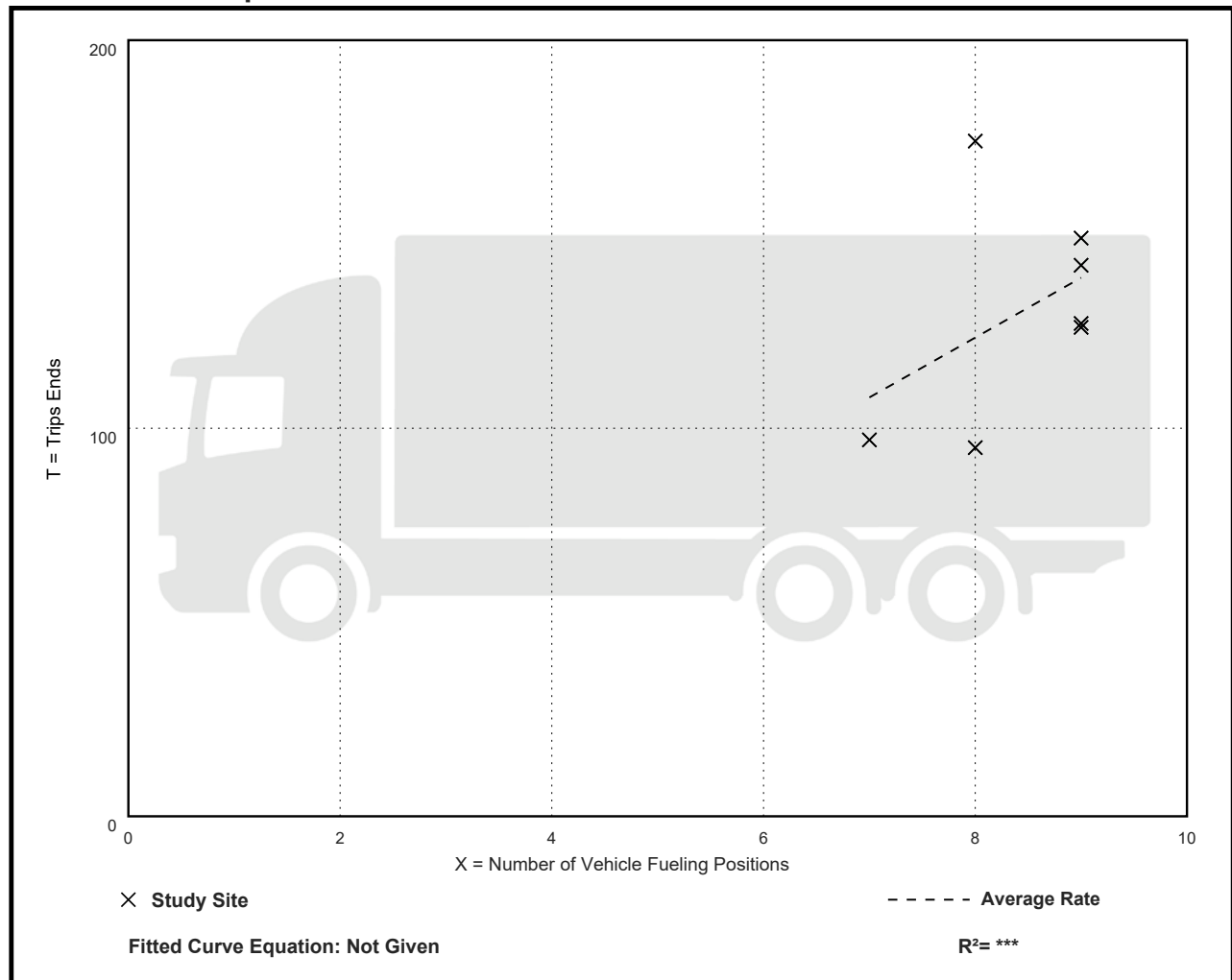
Avg. Num. of Vehicle Fueling Positions: 8

Directional Distribution: 53% entering, 47% exiting

Truck Trip Generation per Vehicle Fueling Position

Average Rate	Range of Rates	Standard Deviation
15.42	11.88 - 21.75	3.10

Data Plot and Equation



September 30, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 950 – Truck Stop

For Weekday → 50% Enter/50% Exit

224.0 x 9 Truck Fuel Position = 2,016 Trips

2,016 Trips x 0.50 (50%) = 1,008 Trips Enter/1,008 Trips Exit

For AM Peak Hour → 49% Enter/51% Exit

13.97 x 9 Truck Fuel Position = 125.73 ≈ 126 Trips for Even Number

Primary Trips = 126 x 0.49 (49%) and 126 x 0.51 (51%) = 62 Trips Enter/64 Trips Exit

For PM Peak Hour → 53% Enter/47% Exit

15.42 x 9 Truck Fuel Position = 138.78 ≈ 138 Trips for Even Number

Primary Trips = 138 x 0.53 (53%) and 138 x 0.47 (47%) = 73 Trips Enter/65 Trips Exit

Land Use: 848

Tire Store

Description

The primary business associated with a tire store is the sale of tires for automotive vehicles. Services offered by these stores usually include tire installation and repair, as well as other automotive maintenance or repair services and customer assistance. These stores generally do not contain large storage or warehouse areas. Automobile parts sales (Land Use 843), tire superstore (Land Use 849), and automobile parts and service center (Land Use 943) are related uses.

Additional Data

The technical appendices provide supporting information on time-of-day distributions for this land use. The appendices can be accessed through either the ITETripGen web app or the trip generation resource page on the ITE website (<https://www.ite.org/technical-resources/topics/trip-and-parking-generation/>).

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Florida, Minnesota, New Jersey, New York, Oregon, Pennsylvania, South Dakota, Texas, and Wisconsin.

Source Numbers

328, 359, 438, 555, 571, 583, 599, 870, 886, 887, 959, 1049

Tire Store (848)

Vehicle Trip Ends vs: Service Bays

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 9

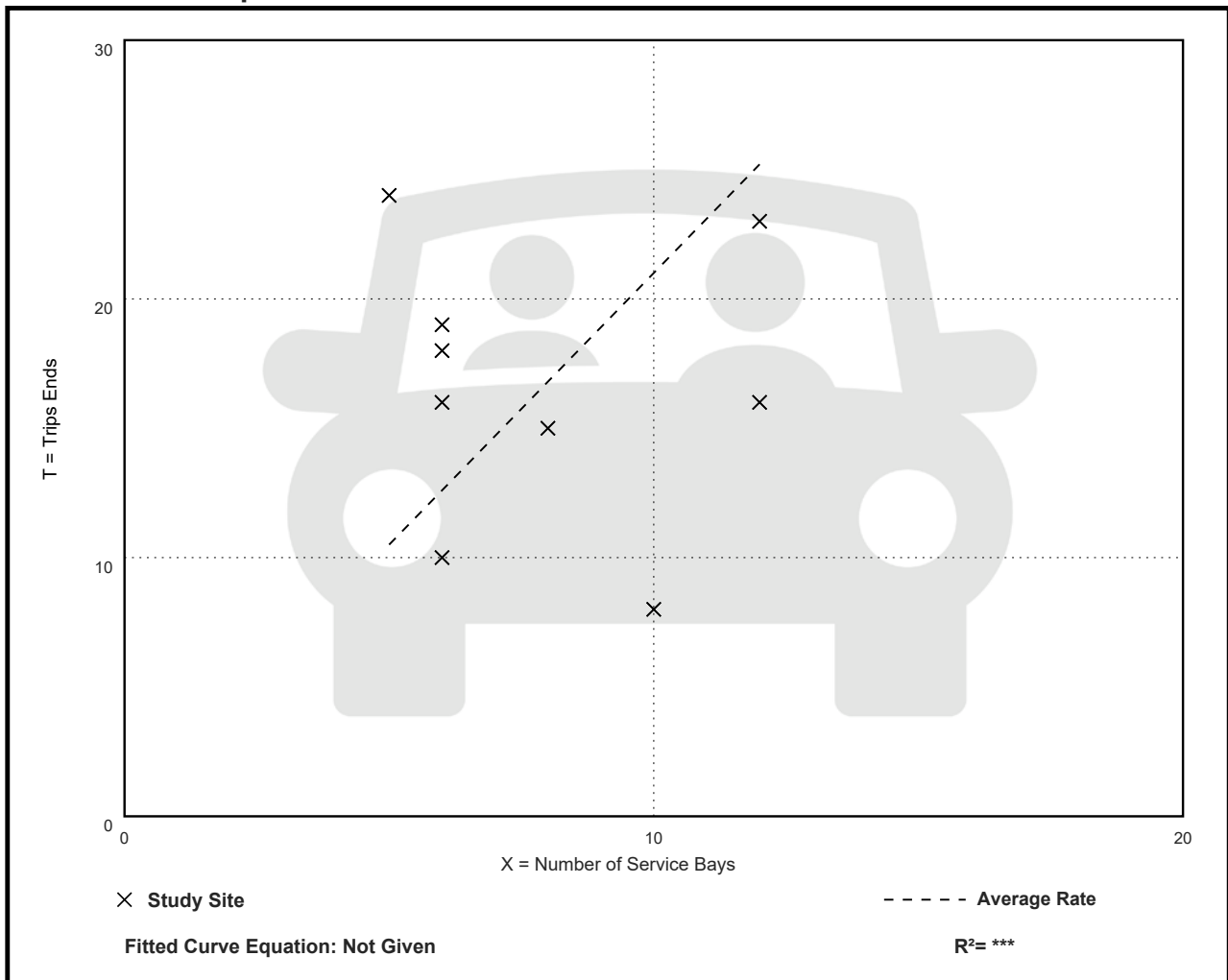
Avg. Num. of Service Bays: 8

Directional Distribution: 64% entering, 36% exiting

Vehicle Trip Generation per Service Bay

Average Rate	Range of Rates	Standard Deviation
2.10	0.80 - 4.80	1.10

Data Plot and Equation



Tire Store (848)

Vehicle Trip Ends vs: Service Bays

On a: **Weekday,**

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 10

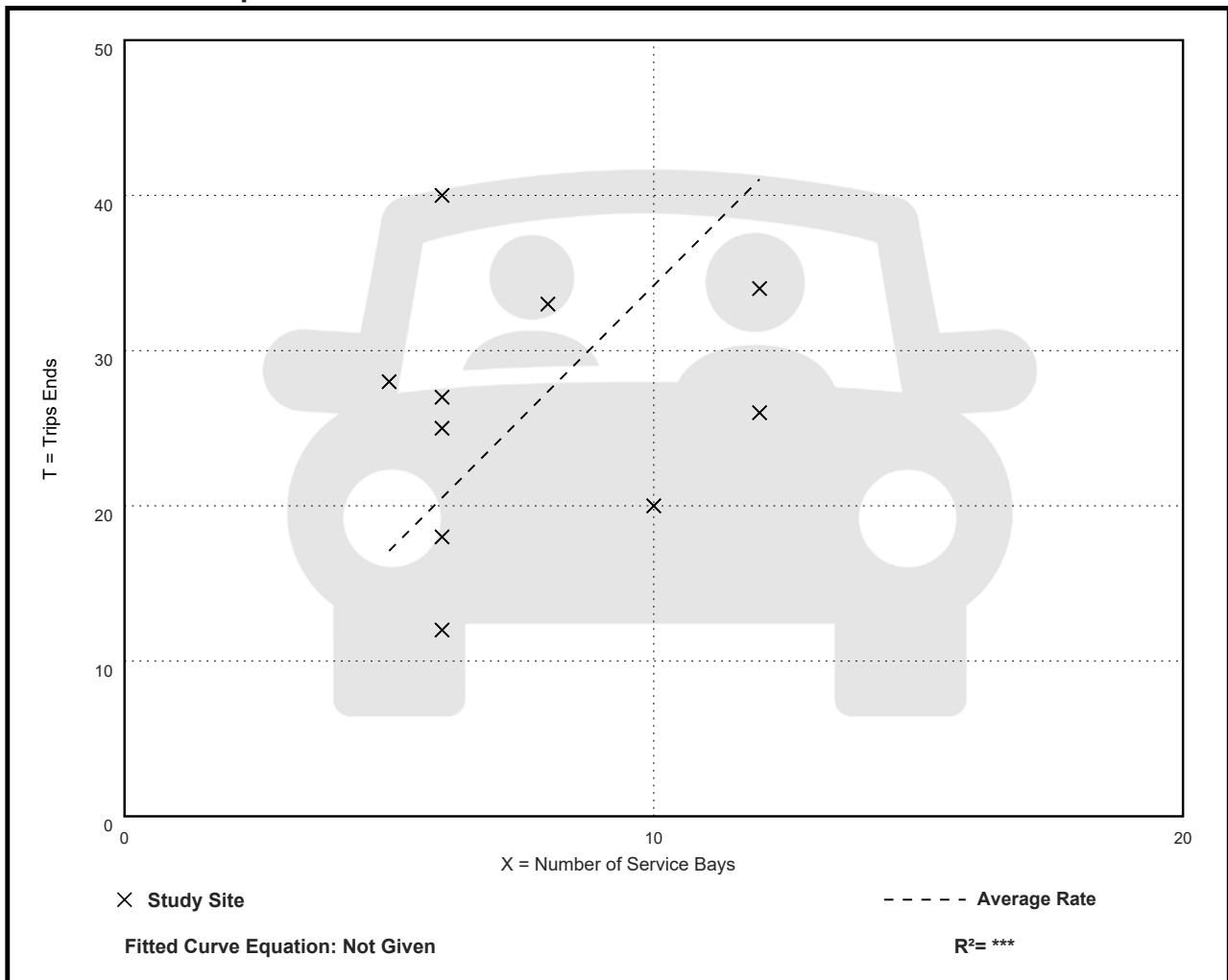
Avg. Num. of Service Bays: 8

Directional Distribution: 42% entering, 58% exiting

Vehicle Trip Generation per Service Bay

Average Rate	Range of Rates	Standard Deviation
3.42	2.00 - 6.67	1.51

Data Plot and Equation



September 30, 2022

Traffic Impact Study – Proposed Love’s Travel Stop

St. Clairsville, Ohio

CESO Trip Generation Calculations – 2023 and 2043 Build Traffic Scenarios:

ITE 848 – Tire Store

For Weekday → 50% Enter/50% Exit

Data N/A.

For AM Peak Hour → 64% Enter/36% Exit

2.10 x 3 Service Bays = 6.3 ≈ **6 Trips** for Even Number

Primary Trips = 6 x 0.64 (64%) and 6 x 0.36 (36%) = **4 Trips Enter/2 Trips Exit**

For PM Peak Hour → 42% Enter/58% Exit

3.42 x 3 Service Bays = 10.26 ≈ **10 Trips**

Primary Trips = 10 x 0.42 (42%) and 10 x 0.58 (58%) = **4 Trips Enter/6 Trips Exit**

NCHRP 684 Internal Trip Capture Estimation Tool					
Project Name:	Love's - St. Clairsville, OH	Organization:	CESO, Inc.		
Project Location:	St. Clairsville, OH	Performed By:	TMC		
Scenario Description:	Build	Date:	9/29/2022		
Analysis Year:	2023 and 2043	Checked By:	REM		
Analysis Period:	AM Street Peak Hour	Date:	9/29/2022		

Table 1-A: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail				564	282	282
Restaurant				263	143	120
Cinema/Entertainment				0		
Residential				0		
Hotel				0		
All Other Land Uses ²				0		
				827	425	402

Table 2-A: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses ²						

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-A: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail	0		37	0	0	0
Restaurant	0	17		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 5-A: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	827	425	402
Internal Capture Percentage	13%	13%	13%
External Vehicle-Trips ⁵	719	371	348
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-A: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	N/A	N/A
Retail	6%	13%
Restaurant	26%	14%
Cinema/Entertainment	N/A	N/A
Residential	N/A	N/A
Hotel	N/A	N/A

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

NCHRP 684 Internal Trip Capture Estimation Tool			
Project Name:	Love's - St. Clairsville, OH	Organization:	CESO, Inc.
Project Location:	St. Clairsville, OH	Performed By:	TMC
Scenario Description:	Build	Date:	9/29/2022
Analysis Year:	2023 and 2043	Checked By:	REM
Analysis Period:	PM Street Peak Hour	Date:	9/29/2022

Table 1-P: Base Vehicle-Trip Generation Estimates (Single-Use Site Estimate)						
Land Use	Development Data (For Information Only)			Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail				513	260	253
Restaurant				198	100	98
Cinema/Entertainment				0		
Residential				0		
Hotel				0		
All Other Land Uses ²				0		
				711	360	351

Table 2-P: Mode Split and Vehicle Occupancy Estimates						
Land Use	Entering Trips			Exiting Trips		
	Veh. Occ. ⁴	% Transit	% Non-Motorized	Veh. Occ. ⁴	% Transit	% Non-Motorized
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						
All Other Land Uses ²						

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel						

Table 4-P: Internal Person-Trip Origin-Destination Matrix*						
Origin (From)	Destination (To)					
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		0	0	0	0	0
Retail	0		29	0	0	0
Restaurant	0	40		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	0	0	0	0		0
Hotel	0	0	0	0	0	

Table 5-P: Computations Summary			
	Total	Entering	Exiting
All Person-Trips	711	360	351
Internal Capture Percentage	19%	19%	20%
External Vehicle-Trips ⁵	573	291	282
External Transit-Trips ⁶	0	0	0
External Non-Motorized Trips ⁶	0	0	0

Table 6-P: Internal Trip Capture Percentages by Land Use		
Land Use	Entering Trips	Exiting Trips
Office	N/A	N/A
Retail	15%	11%
Restaurant	29%	41%
Cinema/Entertainment	N/A	N/A
Residential	N/A	N/A
Hotel	N/A	N/A

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

Love's Development - St. Clairsville, OH
AM PEAK

Passenger Cars

<u>ITE CAT. 933</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	73	52	125
INTERNAL CAPTURE <i>(26% Enter /14% Exit)</i>	19	7	26
	54	45	99
Pass-By (0%)	0	0	0
Primary	54	45	99
<u>ITE CAT. 934</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	70	68	138
INTERNAL CAPTURE <i>(26% Enter /14% Exit)</i>	18	10	28
	52	58	110
Pass-By (50%)	26	28	54
Primary	26	30	56
<u>ITE CAT. 945</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	216	216	432
INTERNAL CAPTURE <i>(6% Enter /13% Exit)</i>	13	28	41
	203	188	391
Pass-By (76%)	154	142	296
Primary	49	46	95

Trucks

<u>ITE CAT. 848</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	4	2	6
INTERNAL CAPTURE <i>(6% Enter /13% Exit)</i>	0	0	0
	4	2	6
Pass-By (0%)	0	0	0
Primary	4	2	6
<u>ITE CAT. 950</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	62	64	126
INTERNAL CAPTURE <i>(6% Enter /13% Exit)</i>	4	8	12
	58	56	114
Pass-By (0%)	0	0	0
Primary	58	56	114

Love's Development - St. Clairsville, OH
PM PEAK

Passenger Cars

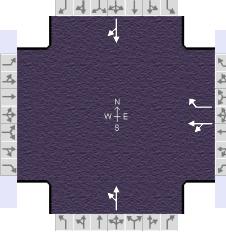
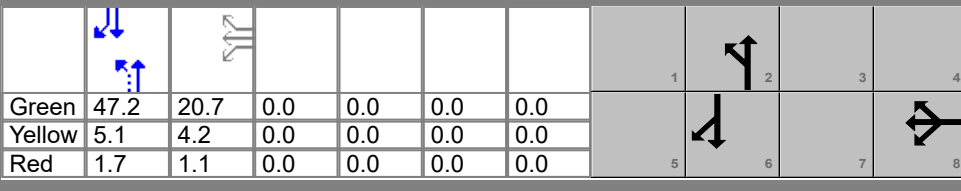
<u>ITE CAT. 933</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	48	48	96
INTERNAL CAPTURE <i>(29% Enter /41% Exit)</i>	14	20	34
	34	28	62
Pass-By (0%)	0	0	0
Primary	34	28	62
<u>ITE CAT. 934</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	52	50	102
INTERNAL CAPTURE <i>(29% Enter /41% Exit)</i>	15	21	36
	37	29	66
Pass-By (55%)	20	16	36
Primary	17	13	30
<u>ITE CAT. 945</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	183	182	365
INTERNAL CAPTURE <i>(15% Enter /11% Exit)</i>	27	20	47
	156	162	318
Pass-By (75%)	118	122	240
Primary	38	40	78

Trucks

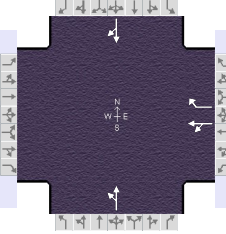
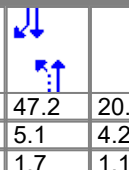
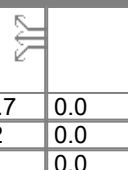
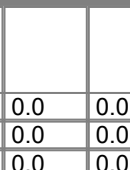
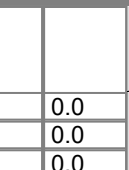
<u>ITE CAT. 848</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	4	6	10
INTERNAL CAPTURE <i>(15% Enter /11% Exit)</i>	1	1	2
	3	5	8
Pass-By (0%)	0	0	0
Primary	3	5	8
<u>ITE CAT. 950</u>	<u>ENTER</u>	<u>EXIT</u>	<u>TOTAL</u>
	73	65	138
INTERNAL CAPTURE <i>(15% Enter /11% Exit)</i>	11	8	19
	62	57	119
Pass-By (0%)	0	0	0
Primary	62	57	119

APPENDIX E
2043 NO-BUILD TRAFFIC SCENARIO
CAPACITY ANALYSIS SUMMARY SHEETS

HCS Signalized Intersection Input Data

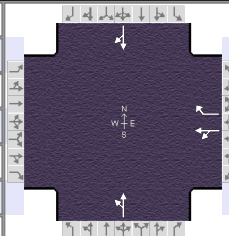
General Information					Intersection Information												
Agency	CESO				Duration, h	0.250											
Analyst	JMP		Analysis Date	Sep 29, 2022		Area Type	Other										
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.81										
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00										
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 NB AM Peak - Int 1 & Int 2.xus													
Project Description	2043 No-Build Traffic Scenario																
Demand Information					EB			WB			NB			SB			
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h								175	1	210	63	294			252	55	
Signal Information																	
Cycle, s	80.0	Reference Phase	2														
Offset, s	0	Reference Point	End														
Uncoordinated	No	Simult. Gap E/W	On														
Force Mode	Fixed	Simult. Gap N/S	On		Green	47.2	20.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
		Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
		Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Traffic Information					EB			WB			NB			SB			
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h								175	1	210	63	294			252	55	
Initial Queue (Q _b), veh/h								0	0	0	0	0			0	0	
Base Saturation Flow Rate (s ₀), veh/h								1750	1750	1750	1750	1750			1750	1750	
Parking (N _m), man/h									None			None			None		
Heavy Vehicles (P _{HV}), %									15	15		8			9		
Ped / Bike / RTOR, /h					0	0		0	0	0	0	0		0	0	0	
Buses (N _b), buses/h								0	0	0	0	0	0	0	0	0	
Arrival Type (AT)								3	3	3	3	3			3	3	
Upstream Filtering (I)								1.00	1.00	1.00	0.95	0.95			1.00	1.00	
Lane Width (W), ft									12.0	12.0		12.0			12.0		
Turn Bay Length, ft									1145	350		400			635		
Grade (P _g), %						0			0			0			0		
Speed Limit, mi/h								35	35	35	45	45			45	45	
Phase Information					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s								26.0		54.0		54.0					
Yellow Change Interval (Y), s								4.2		5.1		5.1					
Red Clearance Interval (R _c), s								1.1		1.7		1.7					
Minimum Green (G _{min}), s								10		20		20					
Start-Up Lost Time (l _t), s							2.0	2.0	2.0	2.0		2.0					
Extension of Effective Green (e), s							2.0	2.0	2.0	2.0		2.0					
Passage (PT), s								2.0		2.0		2.0					
Recall Mode								Off		Min		Min					
Dual Entry								Yes		Yes		Yes					
Walk (Walk), s						0.0		0.0		0.0		0.0					
Pedestrian Clearance Time (PC), s						0.0		0.0		0.0		0.0					
Multimodal Information					EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius					0.0	No	25.0	0.0	No	25.0	0.0	No	25.0				
Walkway / Crosswalk Width / Length, ft					9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0				
Street Width / Island / Curb, ft						0		0.0	0	No	0.0	0	No	0.0		No	
Width Outside / Bike Lane / Shoulder, ft								12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking					No			No	0.50		No	0.50			0.50		

HCS Signalized Intersection Results Summary

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	JMP	Analysis Date	Sep 29, 2022		Area Type	Other										
Jurisdiction	ODOT	Time Period	AM Peak Hour		PHF	0.81										
Urban Street	SR-149	Analysis Year	2043		Analysis Period	1 > 7:00										
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus													
Project Description	2043 No-Build Traffic Scenario															
Demand Information					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h								175	1	210	63	294			252	55
Signal Information																
Cycle, s	80.0	Reference Phase	2		Green	47.2	20.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End		Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On		Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On													
Timer Results					EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase								8		2		6				
Case Number								11.0		8.0		8.0				
Phase Duration, s								26.0		54.0		54.0				
Change Period, (Y+R _c), s								5.3		6.8		6.8				
Max Allow Headway (MAH), s								3.3		0.0		0.0				
Queue Clearance Time (g _s), s								16.6								
Green Extension Time (g _e), s								0.5		0.0		0.0				
Phase Call Probability								1.00								
Max Out Probability								0.63								
Movement Group Results					EB			WB			NB			SB		
Approach Movement					L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement								3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h								217	259		401			379		
Adjusted Saturation Flow Rate (s), veh/h/ln								1472	1310		1457			1576		
Queue Service Time (g _s), s								10.3	14.6		4.8			10.4		
Cycle Queue Clearance Time (g _c), s								10.3	14.6		15.2			10.4		
Green Ratio (g/C)								0.26	0.26		0.59			0.59		
Capacity (c), veh/h								381	339		913			930		
Volume-to-Capacity Ratio (X)								0.570	0.765		0.439			0.408		
Back of Queue (Q), ft/ln (95 th percentile)																
Back of Queue (Q), veh/ln (95 th percentile)								6.4	8.9		8.6			5.6		
Queue Storage Ratio (RQ) (95 th percentile)								0.16	0.71		0.57			0.24		
Uniform Delay (d ₁), s/veh								25.8	27.4		12.8			8.9		
Incremental Delay (d ₂), s/veh								1.3	9.1		1.5			1.3		
Initial Queue Delay (d ₃), s/veh								0.0	0.0		0.0			0.0		
Control Delay (d), s/veh								27.1	36.5		14.3			10.2		
Level of Service (LOS)								C	D		B			B		
Approach Delay, s/veh / LOS					0.0			32.2	C	14.3	B	10.2	B			
Intersection Delay, s/veh / LOS					19.8			B								
Multimodal Results					EB			WB			NB			SB		
Pedestrian LOS Score / LOS					1.72	B		1.72	B	1.65	B	1.36	A			
Bicycle LOS Score / LOS								1.27	A	1.21	A	1.11	A			

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				175	1	210	63	294			252	55

Signal Information														
Cycle, s	80.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	47.2	20.7	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0				
				Red	1.7	1.1	0.0	0.0	0.0	0.0				

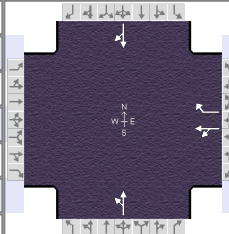
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})				0.953	0.953		0.888	0.888		1.000	0.969	
Right-Turn Adjustment Factor (f_{RT})						0.000	0.847		0.000	0.888		0.969
Left-Turn Pedestrian Adjustment Factor (f_{LPB})				1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})						1.000			1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h				1464	8	1310	257	1200	0	0	1294	282
Proportion of Vehicles Arriving on Green (P)	0.00	0.00	0.00	0.26	0.26	0.26	0.31	0.49	0.00	0.00	0.59	0.59
Incremental Delay Factor (k)					0.10	0.28		0.50			0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)				4.0		6.8		6.8
Green Ratio (g/C)				0.26		0.59		0.59
Permitted Saturation Flow Rate (s_p), veh/h/ln				0		1020		1066
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						0		1627
Permitted Effective Green Time (g_p), s				0.0		47.2		0.0
Permitted Service Time (g_u), s				0.0		36.8		0.0
Permitted Queue Service Time (g_{ps}), s						4.8		
Time to First Blockage (g_t), s				0.0		9.2		47.2
Queue Service Time Before Blockage (g_{ts}), s						9.2		
Protected Right Saturation Flow (s_R), veh/h/ln				0				
Protected Right Effective Green Time (g_R), s				0.0				

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000				
Pedestrian F_s / F_{delay}	0.000	0.148	0.000	0.148	0.000	0.076	0.000	0.076				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		45.16		46.55	1180.00	6.72	1180.00	6.72				
Bicycle F_w / F_v	-3.64		-3.64	0.79	-3.64	0.73	-3.64	0.63				

HCS Signalized Intersection Results Graphical Summary

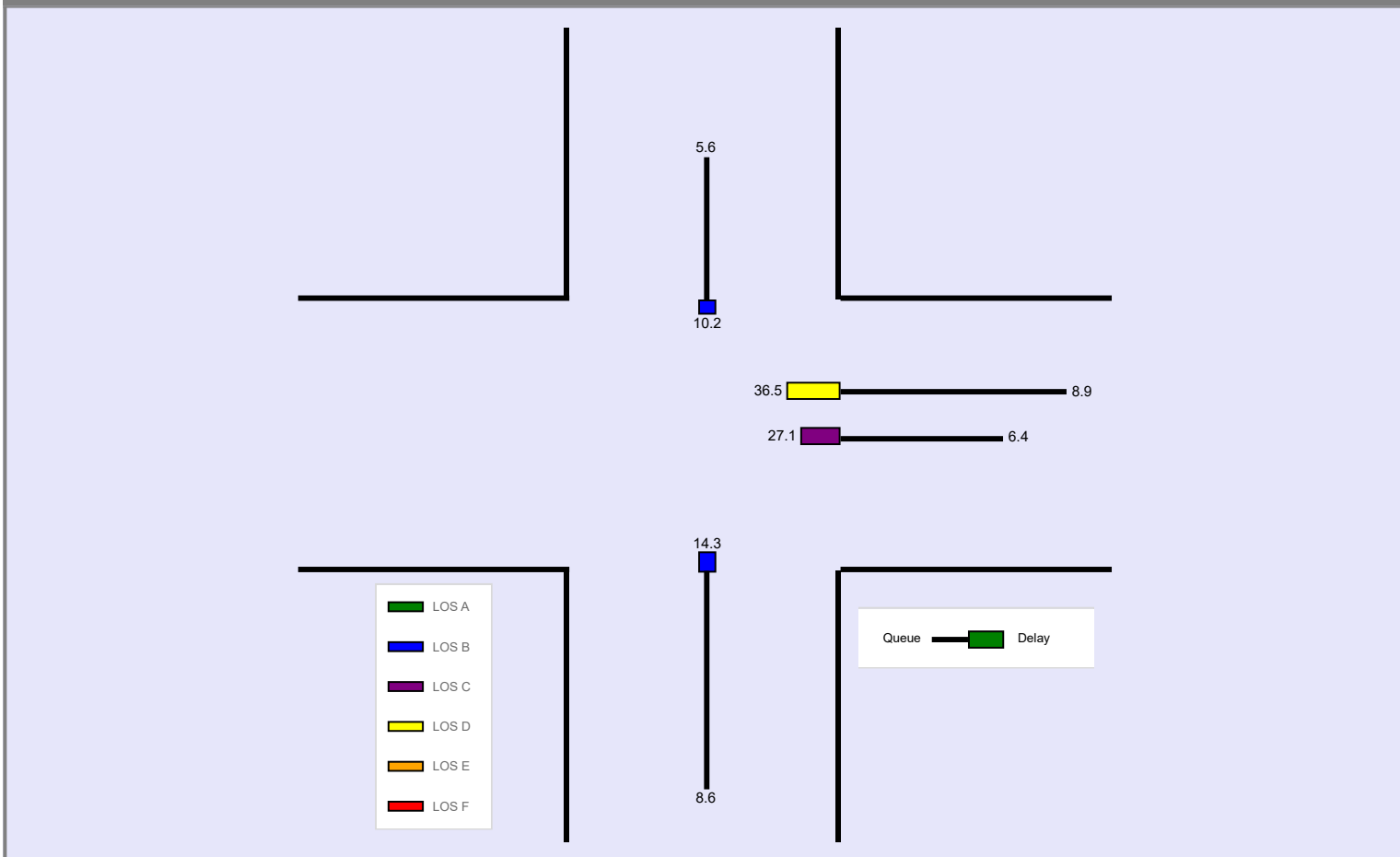
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.81		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				175	1	210	63	294			252	55

Signal Information				Signal Timing (s)									
Cycle, s	80.0	Reference Phase	2	Green	47.2	20.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Offset, s	0	Reference Point	End	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Uncoordinated	No	Simult. Gap E/W	On	Red	1.7	1.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Force Mode	Fixed	Simult. Gap N/S	On										

Movement Group Results	EB			WB			NB			SB			
	L	T	R	L	T	R	L	T	R	L	T	R	
Approach Movement													
Back of Queue (Q), ft/ln (95 th percentile)													
Back of Queue (Q), veh/ln (95 th percentile)					6.4	8.9		8.6			5.6		
Queue Storage Ratio (RQ) (95 th percentile)					0.16	0.71		0.57			0.24		
Control Delay (d), s/veh					27.1	36.5		14.3			10.2		
Level of Service (LOS)					C	D		B			B		
Approach Delay, s/veh / LOS	0.0			32.2	C			14.3	B		10.2	B	
Intersection Delay, s/veh / LOS	19.8						B						

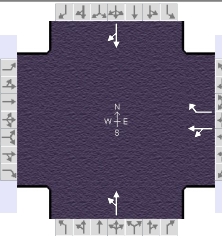


--- Messages ---

No errors or warnings exist.

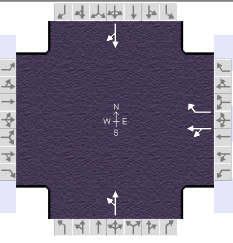
--- Comments ---

HCS Signalized Intersection Input Data

General Information					Intersection Information											
Agency	CESO				Duration, h	0.250										
Analyst	JMP		Analysis Date	Sep 29, 2022		Area Type	Other									
Jurisdiction	ODOT		Time Period	PM Peak Hour		PHF	0.90									
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 4:30									
Intersection	SR-149 & I-70 WB On/O...		File Name	2043 NB PM Peak - Int 1 & Int 2.xus												
Project Description	2043 No-Build Traffic Scenario															
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h							415	1	279	59	339			307	65	
Signal Information							1		2		3		4			
Cycle, s	70.0	Reference Phase	2				5		6		7		8			
Offset, s	0	Reference Point	End				Green		34.2		23.7		0.0			
Uncoordinated	No	Simult. Gap E/W	On				Yellow		5.1		4.2		0.0			
Force Mode	Fixed	Simult. Gap N/S	On		Red		1.7		1.1		0.0					
Traffic Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h							415	1	279	59	339			307	65	
Initial Queue (Q _b), veh/h							0	0	0	0	0			0	0	
Base Saturation Flow Rate (s ₀), veh/h							1750	1750	1750	1750	1750			1750	1750	
Parking (N _m), man/h							None			None			None			
Heavy Vehicles (P _{HV}), %								7	7		9			2		
Ped / Bike / RTOR, /h				0	0		0	0	0	0	0		0	0	0	
Buses (N _b), buses/h							0	0	0	0	0	0	0	0	0	
Arrival Type (AT)							3	3	3	3	3			3	3	
Upstream Filtering (I)							1.00	1.00	1.00	0.94	0.94			1.00	1.00	
Lane Width (W), ft								12.0	12.0		12.0			12.0		
Turn Bay Length, ft								1145	350		400			635		
Grade (P _g), %					0			0			0			0		
Speed Limit, mi/h							35	35	35	45	45			45	45	
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s							29.0		41.0		41.0					
Yellow Change Interval (Y), s							4.2		5.1		5.1					
Red Clearance Interval (R _c), s							1.1		1.7		1.7					
Minimum Green (G _{min}), s							10		20		20					
Start-Up Lost Time (l _t), s						2.0	2.0	2.0	2.0		2.0					
Extension of Effective Green (e), s						2.0	2.0	2.0	2.0		2.0					
Passage (PT), s							2.0		2.0		2.0					
Recall Mode							Off		Min		Min					
Dual Entry							Yes		Yes		Yes					
Walk (Walk), s					0.0		0.0		0.0		0.0					
Pedestrian Clearance Time (PC), s					0.0		0.0		0.0		0.0					
Multimodal Information				EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0	0.0	No	25.0				
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0	9.0	12.0	0.0				
Street Width / Island / Curb, ft					0		0.0	0	No	0.0	0	No	0.0		No	
Width Outside / Bike Lane / Shoulder, ft							12.0	5.0	2.0	12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking				No			No	0.50		No	0.50			0.50		

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				415	1	279	59	339			307	65

Signal Information												
Cycle, s	70.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On	Green	34.2	23.7	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0		
				Red	1.7	1.1	0.0	0.0	0.0	0.0		

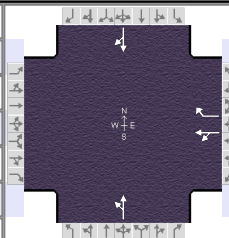
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				8		2		6
Case Number				11.0		8.0		8.0
Phase Duration, s				29.0		41.0		41.0
Change Period, (Y+R _c), s				5.3		6.8		6.8
Max Allow Headway (MAH), s				3.2		0.0		0.0
Queue Clearance Time (g _s), s				21.2				
Green Extension Time (g _e), s				0.6		0.0		0.0
Phase Call Probability				1.00				
Max Out Probability				1.00				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	5	2			6	16
Adjusted Flow Rate (v), veh/h				462	310		428				413	
Adjusted Saturation Flow Rate (s), veh/h/ln				1576	1402		1469				1670	
Queue Service Time (g _s), s				19.2	13.1		2.6				11.8	
Cycle Queue Clearance Time (g _c), s				19.2	13.1		14.4				11.8	
Green Ratio (g/C)				0.34	0.34		0.49				0.49	
Capacity (c), veh/h				534	475		777				816	
Volume-to-Capacity Ratio (X)				0.866	0.653		0.551				0.507	
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)				13.1	7.6		7.4				7.2	
Queue Storage Ratio (RQ) (95 th percentile)				0.30	0.57		0.50				0.29	
Uniform Delay (d ₁), s/veh				21.7	19.7		12.3				12.2	
Incremental Delay (d ₂), s/veh				13.5	2.5		2.6				2.2	
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0				0.0	
Control Delay (d), s/veh				35.1	22.2		14.9				14.4	
Level of Service (LOS)					D	C		B			B	
Approach Delay, s/veh / LOS	0.0			29.9		C	14.9		B	14.4		B
Intersection Delay, s/veh / LOS				22.0			C					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.71	B	1.71	B	1.66	B	1.37	A
Bicycle LOS Score / LOS			1.76	B	1.22	A	1.17	A

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				415	1	279	59	339			307	65

Signal Information														
Cycle, s	70.0	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	No	Simult. Gap E/W	On	Green	34.2	23.7	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0				
				Red	1.7	1.1	0.0	0.0	0.0	0.0				

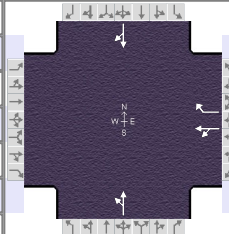
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f _w)												
Heavy Vehicles and Grade Factor (f _{HVg})												
Parking Activity Adjustment Factor (f _p)	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f _{bb})	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f _a)												
Lane Utilization Adjustment Factor (f _{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f _{LT})				0.952	0.952		0.903	0.903		1.000	0.970	
Right-Turn Adjustment Factor (f _{RT})						0.000	0.847		0.000	0.903		0.970
Left-Turn Pedestrian Adjustment Factor (f _{LPB})				1.000			1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f _{Rpb})						1.000			1.000			1.000
Work Zone Adjustment Factor (f _{wz})												
DDI Factor (f _{DDI})												
Movement Saturation Flow Rate (s), veh/h				1572	4	1402	218	1251	0	0	1378	292
Proportion of Vehicles Arriving on Green (P)	0.00	0.00	0.00	0.34	0.34	0.34	0.37	0.52	0.00	0.00	0.49	0.49
Incremental Delay Factor (k)					0.38	0.18		0.50			0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t _L)				4.0		6.8		6.8
Green Ratio (g/C)				0.34		0.49		0.49
Permitted Saturation Flow Rate (s _p), veh/h/ln				0		988		1033
Shared Saturation Flow Rate (s _{sh}), veh/h/ln						0		1723
Permitted Effective Green Time (g _p), s				0.0		34.2		0.0
Permitted Service Time (g _u), s				0.0		22.4		0.0
Permitted Queue Service Time (g _{ps}), s						2.6		
Time to First Blockage (g _t), s				0.0		10.8		34.2
Queue Service Time Before Blockage (g _{ts}), s						10.8		
Protected Right Saturation Flow (s _R), veh/h/ln				0				
Protected Right Effective Green Time (g _R), s				0.0				

Multimodal	EB			WB			NB			SB		
Pedestrian F _w / F _v	0.972	0.000	0.972	0.000	0.972	0.000	0.681	0.000				
Pedestrian F _s / F _{delay}	0.000	0.143	0.000	0.143	0.000	0.089	0.000	0.089				
Pedestrian M _{corner} / M _{cw}	0.00		0.00		0.00		0.00					
Bicycle c _b / d _b		40.18		41.58	977.14	9.15	977.14	9.15				
Bicycle F _w / F _v	-3.64		-3.64	1.27	-3.64	0.73	-3.64	0.68				

HCS Signalized Intersection Results Graphical Summary

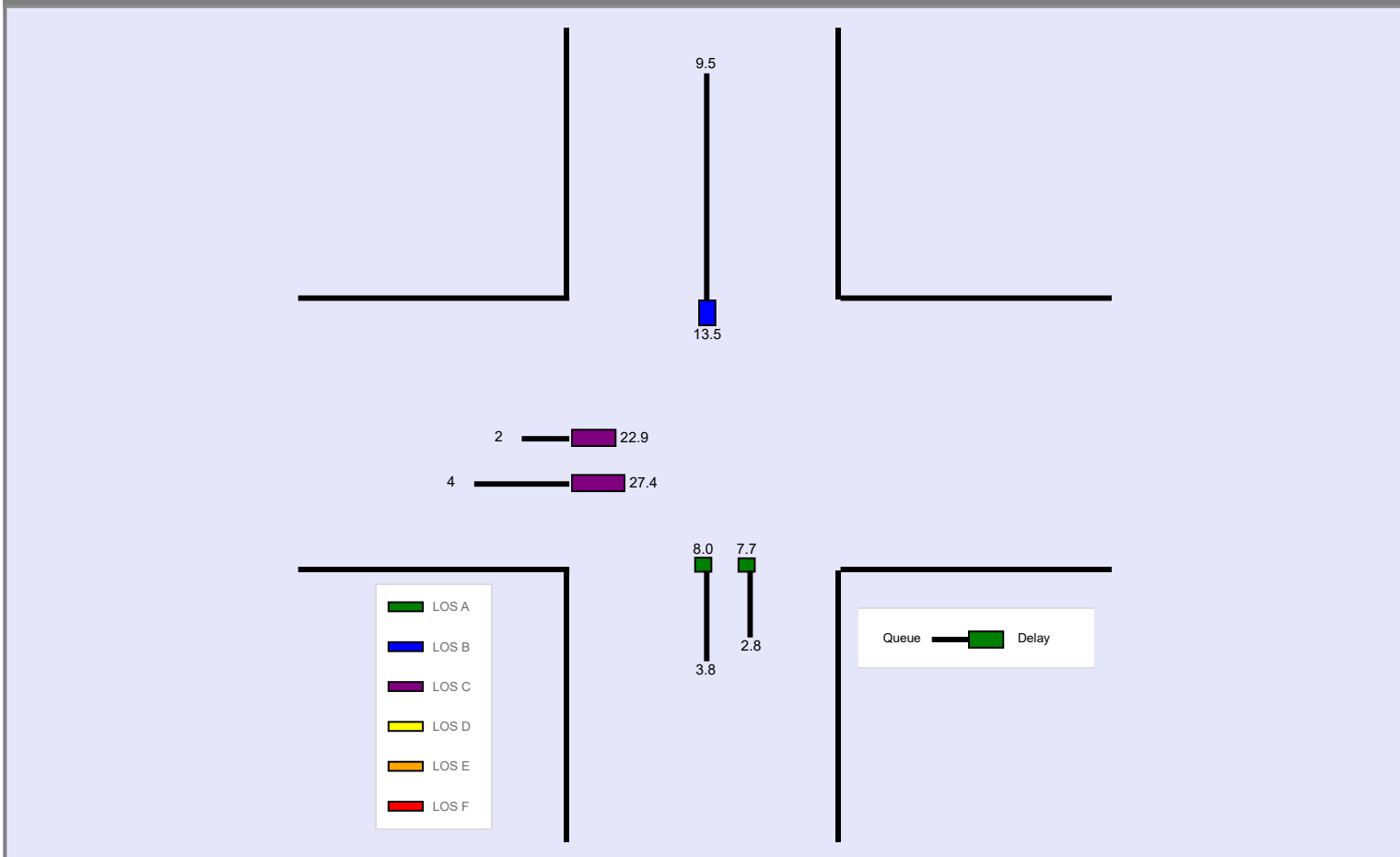
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.90		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 WB On/O...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h				415	1	279	59	339			307	65

Signal Information																		
Cycle, s	70.0	Reference Phase	2															
Offset, s	0	Reference Point	End															
Uncoordinated	No	Simult. Gap E/W	On	Green	34.2	23.7	0.0	0.0	0.0	0.0	1		2		3		4	
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	5.1	4.2	0.0	0.0	0.0	0.0	5		6		7		8	
				Red	1.7	1.1	0.0	0.0	0.0	0.0								

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)						13.1	7.6		7.4			7.2
Queue Storage Ratio (RQ) (95 th percentile)						0.30	0.57		0.50			0.29
Control Delay (d), s/veh						35.1	22.2		14.9			14.4
Level of Service (LOS)						D	C		B			B
Approach Delay, s/veh / LOS	0.0			29.9		C		14.9	B		14.4	B
Intersection Delay, s/veh / LOS	22.0						C					



--- Messages ---

No errors or warnings exist.

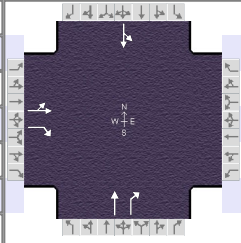
--- Comments ---

HCS Signalized Intersection Input Data

General Information						Intersection Information									
Agency	CESO					Duration, h	0.250								
Analyst	JMP		Analysis Date	Sep 29, 2022		Area Type	Other								
Jurisdiction	ODOT		Time Period	AM Peak Hour		PHF	0.89								
Urban Street	SR-149		Analysis Year	2043		Analysis Period	1 > 7:00								
Intersection	SR-149 & I-70 EB On/Of...		File Name	2043 NB AM Peak - Int 1 & Int 2.xus											
Project Description	2043 No-Build Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				60	3	81				297	353		127	300	
Signal Information															
Cycle, s	80.0	Reference Phase	2	Green	43.6	24.6	0.0	0.0	0.0	0.0					
Offset, s	28	Reference Point	End	Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
Uncoordinated	No	Simult. Gap E/W	On	Red	1.3	1.0	0.0	0.0	0.0	0.0					
Force Mode	Fixed	Simult. Gap N/S	On												
Traffic Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				60	3	81				297	353		127	300	
Initial Queue (Q _b), veh/h				0	0	0				0	0		0	0	
Base Saturation Flow Rate (s ₀), veh/h				1750	1750	1750				1750	1750		1750	1750	
Parking (N _m), man/h				None						None			None		
Heavy Vehicles (P _{HV}), %					19	19					9	9		13	
Ped / Bike / RTOR, /h				0	0	0	0	0		0	0	0	0	0	
Buses (N _b), buses/h				0	0	0				0	0	0	0	0	
Arrival Type (AT)				3	3	3				3	3		3	3	
Upstream Filtering (I)				1.00	1.00	1.00				1.00	1.00		0.88	0.88	
Lane Width (W), ft					12.0	12.0				12.0	12.0		12.0		
Turn Bay Length, ft					1425	250				470	410		400		
Grade (P _g), %					0			0		0			0		
Speed Limit, mi/h				35	35	35				45	45		45	45	
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Maximum Green (G _{max}) or Phase Split, s					30.0				50.0			50.0			
Yellow Change Interval (Y), s					4.4				5.1			5.1			
Red Clearance Interval (R _c), s					1.0				1.3			1.3			
Minimum Green (G _{min}), s					10				20			20			
Start-Up Lost Time (l _t), s				2.0	2.0				2.0	2.0		2.0			
Extension of Effective Green (e), s				2.0	2.0				2.0	2.0		2.0			
Passage (P _T), s					2.0				2.0			2.0			
Recall Mode					Off				Min			Min			
Dual Entry					Yes				Yes			Yes			
Walk (Walk), s					0.0		0.0					0.0			
Pedestrian Clearance Time (P _C), s					0.0		0.0					0.0			
Multimodal Information				EB			WB			NB			SB		
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0				0.0	No	25.0
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0
Street Width / Island / Curb, ft				0.0	0	No		0		0.0		No	0.0	0	No
Width Outside / Bike Lane / Shoulder, ft				12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0
Pedestrian Signal / Occupied Parking				No	0.50		No				0.50		No	0.50	

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	81						297	353	127	300

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	28	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	43.6	24.6	0.0	0.0	0.0	0.0				
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0				
		Red	1.3	1.0	0.0	0.0	0.0	0.0				

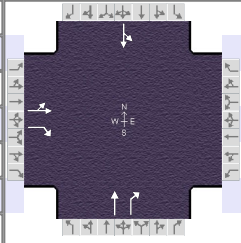
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4				2		6
Case Number		11.0				7.0		8.0
Phase Duration, s		30.0				50.0		50.0
Change Period, (Y+R _c), s		5.4				6.4		6.4
Max Allow Headway (MAH), s		3.3				0.0		0.0
Queue Clearance Time (g _s), s		6.3						
Green Extension Time (g _e), s		0.3				0.0		0.0
Phase Call Probability		1.00						
Max Out Probability		0.00						

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14					2	12	1	6	
Adjusted Flow Rate (v), veh/h		71	91					334	397		527	
Adjusted Saturation Flow Rate (s), veh/h/ln		1423	1263					1627	1379		1201	
Queue Service Time (g _s), s		2.9	4.3					9.4	14.7		21.8	
Cycle Queue Clearance Time (g _c), s		2.9	4.3					9.4	14.7		31.2	
Green Ratio (g/C)		0.31	0.31					0.55	0.55		0.55	
Capacity (c), veh/h		438	388					887	752		713	
Volume-to-Capacity Ratio (X)		0.162	0.234					0.376	0.528		0.739	
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		1.7	2.2					5.5	7.5		12.6	
Queue Storage Ratio (RQ) (95 th percentile)		0.03	0.25					0.31	0.49		0.87	
Uniform Delay (d ₁), s/veh		20.2	20.7					10.4	11.6		17.4	
Incremental Delay (d ₂), s/veh		0.1	0.1					1.2	2.6		6.0	
Initial Queue Delay (d ₃), s/veh		0.0	0.0					0.0	0.0		0.0	
Control Delay (d), s/veh		20.3	20.8					11.6	14.3		23.4	
Level of Service (LOS)		C	C					B	B		C	
Approach Delay, s/veh / LOS	20.6	C		0.0			13.1	B		23.4	C	
Intersection Delay, s/veh / LOS	17.7						B					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.95	B	1.72	B	1.37	A	1.66	B
Bicycle LOS Score / LOS	0.75	A			1.69	B	1.28	A

HCS Signalized Intersection Intermediate Values

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	81					297	353	127	300	

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	28	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	43.6	24.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Red	1.3	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

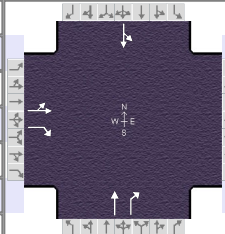
Saturation Flow / Delay	L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f_w)												
Heavy Vehicles and Grade Factor (f_{HVg})												
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f_a)												
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f_{LT})	0.955	0.955					1.000	1.000		0.764	0.764	
Right-Turn Adjustment Factor (f_{RT})		0.000	0.847					0.000	0.847		0.000	0.764
Left-Turn Pedestrian Adjustment Factor (f_{LPB})	1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f_{RPB})			1.000						1.000			1.000
Work Zone Adjustment Factor (f_{wz})												
DDI Factor (f_{DDI})												
Movement Saturation Flow Rate (s), veh/h	1355	68	1263				0	1627	1379	357	844	0
Proportion of Vehicles Arriving on Green (P)	0.31	0.31	0.31	0.00	0.00	0.00	0.00	0.55	0.55	0.62	0.47	0.00
Incremental Delay Factor (k)		0.04	0.04					0.50	0.50		0.50	

Signal Timing / Movement Groups	EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R
Lost Time (t_L)		4.0				6.4		6.4
Green Ratio (g/C)		0.31				0.55		0.55
Permitted Saturation Flow Rate (s_p), veh/h/ln		0				1028		1063
Shared Saturation Flow Rate (s_{sh}), veh/h/ln						1627		0
Permitted Effective Green Time (g_p), s		0.0				0.0		43.6
Permitted Service Time (g_u), s		0.0				0.0		34.2
Permitted Queue Service Time (g_{ps}), s								21.8
Time to First Blockage (g_f), s		0.0				43.6		4.6
Queue Service Time Before Blockage (g_{ts}), s								4.6
Protected Right Saturation Flow (s_R), veh/h/ln		0				0		
Protected Right Effective Green Time (g_R), s		0.0				0.0		

Multimodal	EB			WB			NB			SB		
Pedestrian F_w / F_v	1.198	0.000	0.972	0.000	0.681	0.000	0.972	0.000				
Pedestrian F_s / F_{delay}	0.000	0.148	0.000	0.148	0.000	0.085	0.000	0.085				
Pedestrian M_{corner} / M_{cw}	0.00		0.00		0.00		0.00					
Bicycle c_b / d_b		46.66		45.16	1090.00	8.28	1090.00	8.28				
Bicycle F_w / F_v	-3.64	0.27	-3.64		-3.64	1.21	-3.64	0.79				

HCS Signalized Intersection Results Graphical Summary

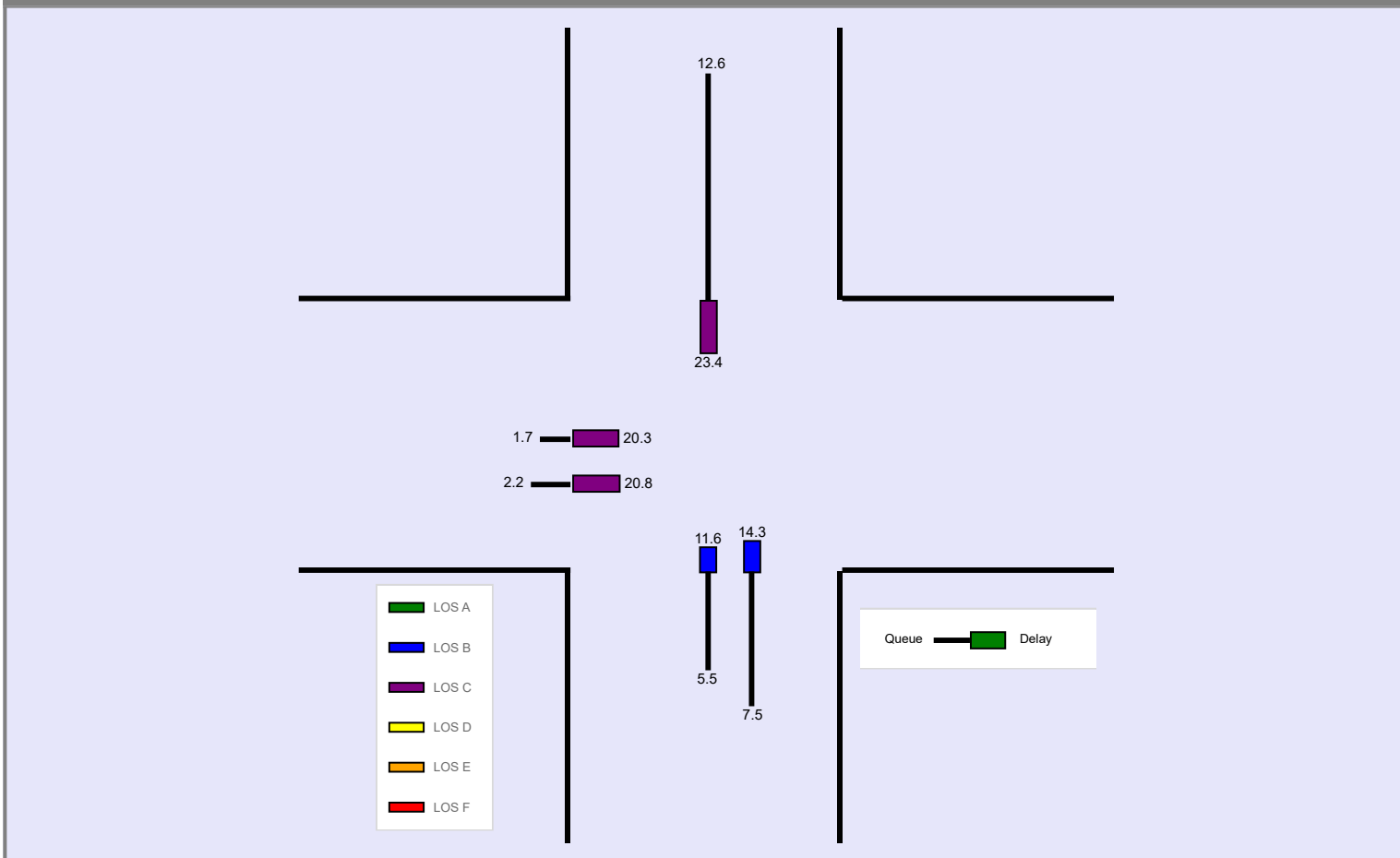
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	AM Peak Hour	PHF	0.89		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 7:00		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB AM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	60	3	81					297	353	127	300	

Signal Information												
Cycle, s	80.0	Reference Phase	2									
Offset, s	28	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	43.6	24.6	0.0	0.0	0.0	0.0				
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0				
		Red	1.3	1.0	0.0	0.0	0.0	0.0				

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		1.7	2.2					5.5	7.5		12.6	
Queue Storage Ratio (RQ) (95 th percentile)		0.03	0.25					0.31	0.49		0.87	
Control Delay (d), s/veh		20.3	20.8					11.6	14.3		23.4	
Level of Service (LOS)		C	C					B	B		C	
Approach Delay, s/veh / LOS	20.6	C		0.0			13.1	B		23.4	C	
Intersection Delay, s/veh / LOS	17.7						B					



--- Messages ---

No errors or warnings exist.

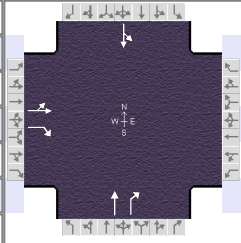
--- Comments ---

HCS Signalized Intersection Input Data

General Information						Intersection Information										
Agency		CESO				Duration, h		0.250								
Analyst		JMP		Analysis Date		Sep 29, 2022		Area Type		Other						
Jurisdiction		ODOT		Time Period		PM Peak Hour		PHF		0.93						
Urban Street		SR-149		Analysis Year		2043		Analysis Period		1 > 4:30						
Intersection		SR-149 & I-70 EB On/Of...		File Name		2043 NB PM Peak - Int 1 & Int 2.xus										
Project Description		2043 No-Build Traffic Scenario														
Demand Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				75	4	136				323	242		89	633		
Signal Information																
Cycle, s		70.0		Reference Phase		2										
Offset, s		38		Reference Point		End										
Uncoordinated		No		Simult. Gap E/W		On		Green		42.6		15.6		0.0		
Force Mode		Fixed		Simult. Gap N/S		On		Yellow		5.1		4.4		0.0		
								Red		1.3		1.0		0.0		
Traffic Information				EB			WB			NB			SB			
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R	
Demand (v), veh/h				75	4	136				323	242		89	633		
Initial Queue (Q _b), veh/h				0	0	0				0	0		0	0		
Base Saturation Flow Rate (s ₀), veh/h				1750	1750	1750				1750	1750		1750	1750		
Parking (N _m), man/h				None						None			None			
Heavy Vehicles (P _{HV}), %				34		34					13		13		7	
Ped / Bike / RTOR, /h				0	0	0	0	0		0	0	0	0	0		
Buses (N _b), buses/h				0	0	0				0	0	0	0	0		
Arrival Type (AT)				3	3	3				3	3		3	3		
Upstream Filtering (I)				1.00	1.00	1.00				1.00	1.00		0.63	0.63		
Lane Width (W), ft				12.0		12.0					12.0		12.0			
Turn Bay Length, ft				1425		250					470		410			
Grade (Pg), %				0				0			0		0			
Speed Limit, mi/h				35	35	35				45	45		45	45		
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT					
Maximum Green (G _{max}) or Phase Split, s					21.0				49.0		49.0					
Yellow Change Interval (Y), s					4.4				5.1		5.1					
Red Clearance Interval (R _c), s					1.0				1.3		1.3					
Minimum Green (G _{min}), s					10				20		20					
Start-Up Lost Time (lt), s				2.0	2.0				2.0	2.0	2.0					
Extension of Effective Green (e), s				2.0	2.0				2.0	2.0	2.0					
Passage (PT), s					2.0				2.0		2.0					
Recall Mode					Off				Min		Min					
Dual Entry					Yes				Yes		Yes					
Walk (Walk), s					0.0		0.0				0.0					
Pedestrian Clearance Time (PC), s					0.0		0.0				0.0					
Multimodal Information				EB			WB			NB			SB			
85th % Speed / Rest in Walk / Corner Radius				0.0	No	25.0	0.0	No	25.0				0.0	No	25.0	
Walkway / Crosswalk Width / Length, ft				9.0	12.0	0.0	9.0	12.0	0.0				9.0	12.0	0.0	
Street Width / Island / Curb, ft				0.0	0	No		0		0.0		No	0.0	0	No	
Width Outside / Bike Lane / Shoulder, ft				12.0	5.0	2.0				12.0	5.0	2.0	12.0	5.0	2.0	
Pedestrian Signal / Occupied Parking				No	0.50		No				0.50		No	0.50		

HCS Signalized Intersection Results Summary

General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand (v), veh/h	75	4	136					323	242	89	633	

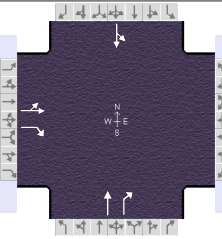
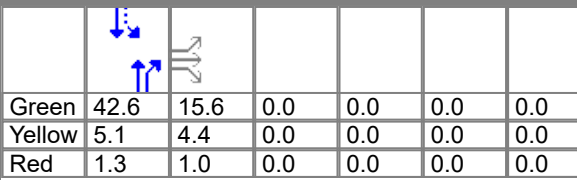
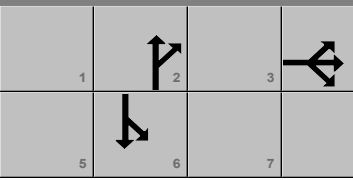
Signal Information												
Cycle, s	70.0	Reference Phase	2									
Offset, s	38	Reference Point	End									
Uncoordinated	No	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	42.6	15.6	0.0	0.0	0.0	0.0				
		Yellow	5.1	4.4	0.0	0.0	0.0	0.0				
		Red	1.3	1.0	0.0	0.0	0.0	0.0				

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4				2		6
Case Number		11.0				7.0		8.0
Phase Duration, s		21.0				49.0		49.0
Change Period, ($Y+R_c$), s		5.4				6.4		6.4
Max Allow Headway (MAH), s		3.3				0.0		0.0
Queue Clearance Time (g_s), s		10.4						
Green Extension Time (g_e), s		0.2				0.0		0.0
Phase Call Probability		1.00						
Max Out Probability		0.19						

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Assigned Movement	7	4	14				2	12		1	6	
Adjusted Flow Rate (v), veh/h		85	146				347	260			802	
Adjusted Saturation Flow Rate (s), veh/h/ln		1228	1090				1573	1333			1557	
Queue Service Time (g_s), s		4.0	8.4				7.8	6.6			17.7	
Cycle Queue Clearance Time (g_c), s		4.0	8.4				7.8	6.6			27.3	
Green Ratio (g/C)		0.22	0.22				0.61	0.61			0.61	
Capacity (c), veh/h		274	243				957	811			1005	
Volume-to-Capacity Ratio (X)		0.310	0.602				0.363	0.321			0.798	
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)		2.0	4.0				3.8	2.8			9.5	
Queue Storage Ratio (RQ) (95 th percentile)		0.04	0.51				0.22	0.19			0.63	
Uniform Delay (d_1), s/veh		22.7	24.4				6.9	6.7			9.3	
Incremental Delay (d_2), s/veh		0.2	3.0				1.1	1.0			4.3	
Initial Queue Delay (d_3), s/veh		0.0	0.0				0.0	0.0			0.0	
Control Delay (d), s/veh		22.9	27.4				8.0	7.7			13.5	
Level of Service (LOS)		C	C				A	A			B	
Approach Delay, s/veh / LOS	25.7	C		0.0			7.8	A		13.5	B	
Intersection Delay, s/veh / LOS	13.1						B					

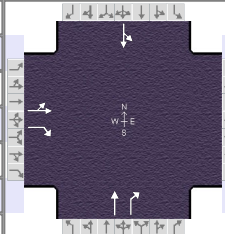
Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.94	B	1.71	B	1.35	A	1.64	B
Bicycle LOS Score / LOS	0.87	A			1.49	A	1.77	B

HCS Signalized Intersection Intermediate Values

General Information					Intersection Information										
Agency	CESO				Duration, h	0.250									
Analyst	JMP	Analysis Date	Sep 29, 2022		Area Type	Other									
Jurisdiction	ODOT	Time Period	PM Peak Hour		PHF	0.93									
Urban Street	SR-149	Analysis Year	2043		Analysis Period	1 > 4:30									
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus												
Project Description	2043 No-Build Traffic Scenario														
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				75	4	136				323	242		89	633	
Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	38	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On	Green	42.6	15.6	0.0	0.0	0.0	0.0					
				Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
				Red	1.3	1.0	0.0	0.0	0.0	0.0					
Saturation Flow / Delay				L	T	R	L	T	R	L	T	R	L	T	R
Lane Width Adjustment Factor (f _w)															
Heavy Vehicles and Grade Factor (f _{HVg})															
Parking Activity Adjustment Factor (f _p)				1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Bus Blockage Adjustment Factor (f _{bb})				1.000	1.000	1.000	0.000	0.000	0.000	1.000	1.000	1.000	1.000	1.000	1.000
Area Type Adjustment Factor (f _a)															
Lane Utilization Adjustment Factor (f _{LU})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Left-Turn Adjustment Factor (f _{LT})				0.955	0.955					1.000	1.000		0.941	0.941	
Right-Turn Adjustment Factor (f _{RT})					0.000	0.847					0.000	0.847		0.000	0.941
Left-Turn Pedestrian Adjustment Factor (f _{LPB})				1.000						1.000			1.000		
Right-Turn Ped-Bike Adjustment Factor (f _{RPB})						1.000						1.000			1.000
Work Zone Adjustment Factor (f _{wz})															
DDI Factor (f _{DDI})															
Movement Saturation Flow Rate (s), veh/h				1165	62	1090				0	1573	1333	192	1365	0
Proportion of Vehicles Arriving on Green (P)				0.22	0.22	0.22	0.00	0.00	0.00	0.00	0.61	0.61	0.66	0.65	0.00
Incremental Delay Factor (k)					0.04	0.13					0.50	0.50		0.50	
Signal Timing / Movement Groups				EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R				
Lost Time (t _L)					4.0				6.4		6.4				
Green Ratio (g/C)					0.22				0.61		0.61				
Permitted Saturation Flow Rate (s _p), veh/h/ln					0				756		1050				
Shared Saturation Flow Rate (s _{sh}), veh/h/ln									1573		0				
Permitted Effective Green Time (g _p), s					0.0				0.0		42.6				
Permitted Service Time (g _u), s					0.0				0.0		34.8				
Permitted Queue Service Time (g _{ps}), s											17.7				
Time to First Blockage (g _t), s					0.0				42.6		9.6				
Queue Service Time Before Blockage (g _{ts}), s											9.6				
Protected Right Saturation Flow (s _R), veh/h/ln					0				0						
Protected Right Effective Green Time (g _R), s					0.0				0.0						
Multimodal				EB			WB			NB			SB		
Pedestrian F _w / F _v				1.198	0.000	0.972	0.000	0.681	0.000	0.972	0.000				
Pedestrian F _s / F _{delay}				0.000	0.143	0.000	0.143	0.000	0.067	0.000	0.067				
Pedestrian M _{corner} / M _{cw}				0.00		0.00		0.00		0.00					
Bicycle c _b / d _b					41.69		40.18	1217.14	5.36	1217.14	5.36				
Bicycle F _w / F _v				-3.64	0.38	-3.64		-3.64	1.00	-3.64	1.28				

HCS Signalized Intersection Results Graphical Summary

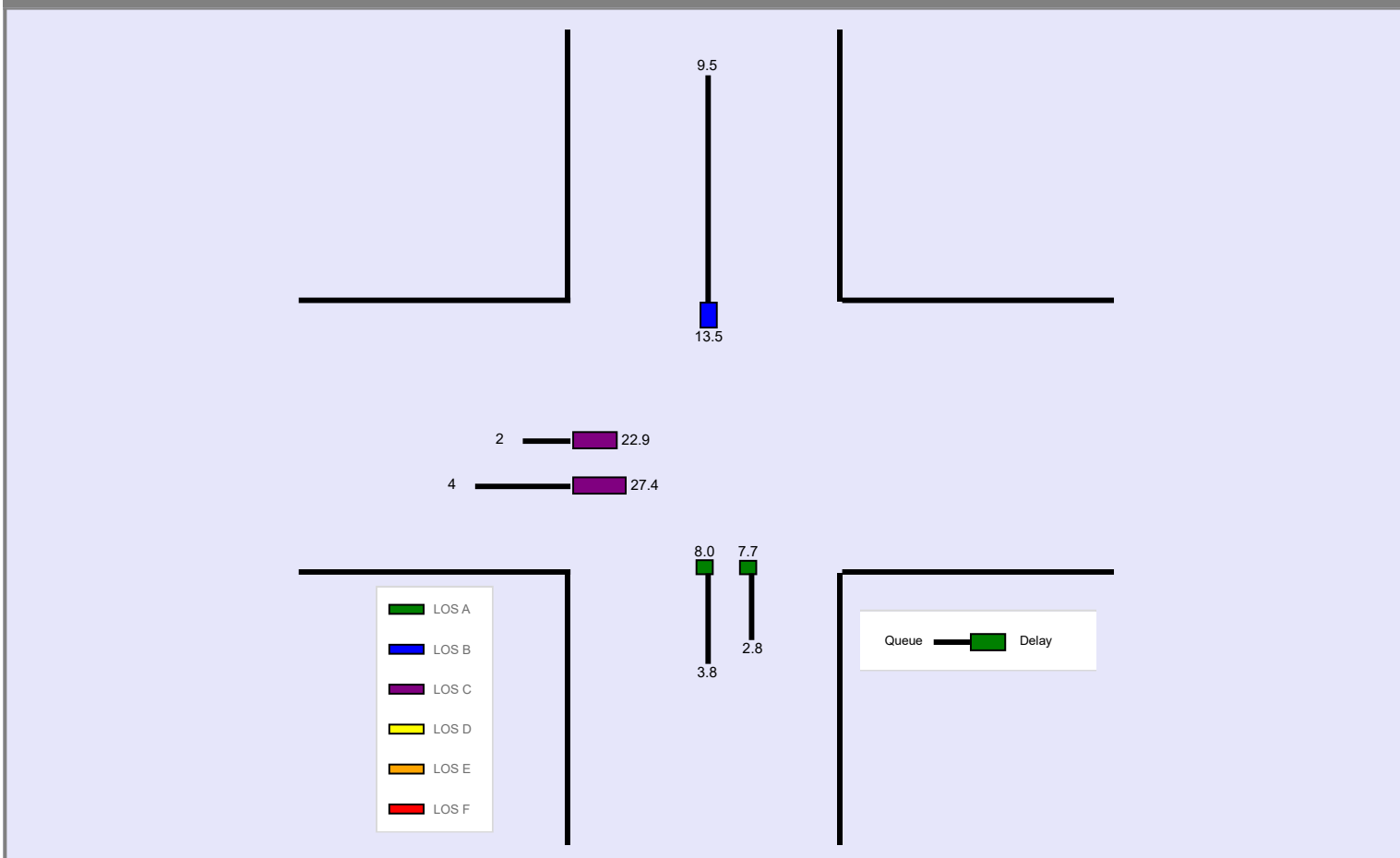
General Information				Intersection Information			
Agency	CESO			Duration, h	0.250		
Analyst	JMP	Analysis Date	Sep 29, 2022	Area Type	Other		
Jurisdiction	ODOT	Time Period	PM Peak Hour	PHF	0.93		
Urban Street	SR-149	Analysis Year	2043	Analysis Period	1 > 4:30		
Intersection	SR-149 & I-70 EB On/Of...	File Name	2043 NB PM Peak - Int 1 & Int 2.xus				
Project Description	2043 No-Build Traffic Scenario						



Demand Information	EB			WB			NB			SB			
	L	T	R	L	T	R	L	T	R	L	T	R	
Approach Movement													
Demand (v), veh/h	75	4	136							323	242	89	633

Signal Information															
Cycle, s	70.0	Reference Phase	2												
Offset, s	38	Reference Point	End												
Uncoordinated	No	Simult. Gap E/W	On												
Force Mode	Fixed	Simult. Gap N/S	On												
				Green	42.6	15.6	0.0	0.0	0.0	0.0					
				Yellow	5.1	4.4	0.0	0.0	0.0	0.0					
				Red	1.3	1.0	0.0	0.0	0.0	0.0					

Movement Group Results	EB			WB			NB			SB			
	L	T	R	L	T	R	L	T	R	L	T	R	
Approach Movement													
Back of Queue (Q), ft/ln (95 th percentile)													
Back of Queue (Q), veh/ln (95 th percentile)		2.0	4.0					3.8	2.8			9.5	
Queue Storage Ratio (RQ) (95 th percentile)		0.04	0.51					0.22	0.19			0.63	
Control Delay (d), s/veh		22.9	27.4					8.0	7.7			13.5	
Level of Service (LOS)		C	C					A	A			B	
Approach Delay, s/veh / LOS	25.7		C			0.0		7.8	A			13.5	B
Intersection Delay, s/veh / LOS	13.1						B						



--- Messages ---

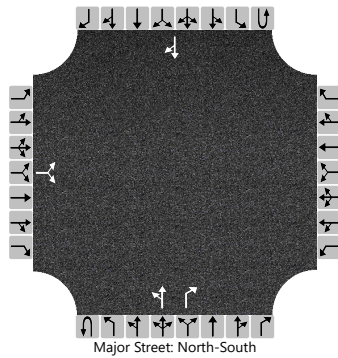
No errors or warnings exist.

--- Comments ---

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP			Intersection	SR-149 & Reco Drive		
Agency/Co.	CESO			Jurisdiction	ODOT		
Date Performed	10/18/2022			East/West Street	Reco Drive		
Analysis Year	2043			North/South Street	SR-149		
Time Analyzed	AM Peak Hour			Peak Hour Factor	0.90		
Intersection Orientation	North-South			Analysis Time Period (hrs)	0.25		
Project Description	No-Build						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0	0	0	1	1	0	0	1	0	
Configuration			LR							LT		R				TR	
Volume (veh/h)		7		9						13	290	353				345	36
Percent Heavy Vehicles (%)		6		6						9							
Proportion Time Blocked																	
Percent Grade (%)		0															
Right Turn Channelized										No							
Median Type Storage		Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1						
Critical Headway (sec)		7.16		6.26						4.19						
Base Follow-Up Headway (sec)		3.5		3.3						2.2						
Follow-Up Headway (sec)		3.55		3.35						2.28						

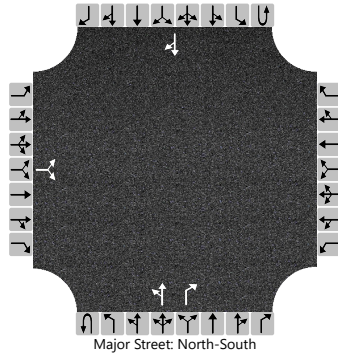
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			18							14						
Capacity, c (veh/h)			441							1099						
v/c Ratio			0.04							0.01						
95% Queue Length, Q ₉₅ (veh)			0.1							0.0						
Control Delay (s/veh)			13.5							8.3	0.1					
Level of Service (LOS)			B							A	A					
Approach Delay (s/veh)		13.5								0.2						
Approach LOS		B								A						

HCS Two-Way Stop-Control Report

General Information				Site Information			
Analyst	JMP	Intersection	SR-149 & Reco Drive				
Agency/Co.	CESO	Jurisdiction	ODOT				
Date Performed	10/18/2022	East/West Street	Reco Drive				
Analysis Year	2043	North/South Street	SR-149				
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.93				
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25				
Project Description	No-Build						

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0		0	1	1		0	0	1	0
Configuration			LR							LT		R					TR
Volume (veh/h)		23		10						9	300	242				760	9
Percent Heavy Vehicles (%)		6		6						13							
Proportion Time Blocked																	
Percent Grade (%)	0																
Right Turn Channelized									No								
Median Type Storage	Undivided																

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1		6.2						4.1							
Critical Headway (sec)		7.16		6.26						4.23							
Base Follow-Up Headway (sec)		3.5		3.3						2.2							
Follow-Up Headway (sec)		3.55		3.35						2.32							

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			35							10							
Capacity, c (veh/h)			199							758							
v/c Ratio			0.18							0.01							
95% Queue Length, Q ₉₅ (veh)			0.6							0.0							
Control Delay (s/veh)			27.0							9.8	0.1						
Level of Service (LOS)			D							A	A						
Approach Delay (s/veh)	27.0								0.2								
Approach LOS	D								A								



11.21.2022
Revised: 01.09.2023

Traffic Impact Study

Proposed Love's Development



On behalf of:



Contact:

Love's Travel Stops
Rick Shuffield
10601 N. Pennsylvania Ave.
Oklahoma City, OK 73120

Preparation Date:

01/09/2023

Traffic Study

CLIENT Love's Travel Stops & Country Stores

STORE # _____

LOCATION: Union Township, Ohio

ADDRESS Southwest Corner of Reco Drive & SR-149

COUNTY Belmont

CITY, STATE Union Township, OH

PREPARED BY Taylor Cline

CESO, Inc.

ADDRESS 175 Montrose West Ave, Suite 400

CITY, STATE Akron, OH 44321

PHONE 330.665.0660

DATE January 9, 2023



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1. Executive Summary

1.1. Summary

This report is submitted on behalf of Love’s Travel Stops in connection with its application to Union Township, Ohio (OH) for Site Plan approval. The Traffic Impact Study (TIS) conducted by CESO, Inc. addresses the traffic related impacts associated with the proposed Love’s Travel Stop; referred to herein as “Love’s Development.”

The proposed Love’s Development is to be located in the southwest corner of SR-149 and Reco Drive, within Union Township, Belmont County, OH. The full buildout of the Love’s Development is projected to have a 11,300 S.F. convenience market with 25 fueling stations (16 passenger car and 9 truck), a 3,100 S.F. fast-food restaurant with a drive-through window, a 2,900 S.F. fast-food restaurant without a drive-through window, a 3-bay tire store, and Campground/Recreational Vehicle Park with 70 campsites. The Love’s Development is projected to have approximately 104 parking spaces for passenger cars, 97 parking spaces for semi-trucks and 70 total RV parking spaces.

Love’s Site Plan application requests approval of the following access points:

- Full access auto driveway connection to Reco Drive (referred to as “Love’s Site Driveway #1”) approximately 340’ west of SR-149 (centerline-to-centerline).
- Full access RV and truck driveway connection to SR-149 (referred to as “Love’s Site Driveway #2”) approximately 670’ south of Reco Drive (centerline-to-centerline).

The Traffic Impact Study focused on evaluating the 2043 No-Build and 2043 Design Year Condition 1/Condition 2 traffic conditions near the site.

1.2. Conclusions

The full buildout of the Love’s Travel Stop is estimated to generate 8,886 trips per day on a typical weekday (4,443 inbound and 4,443 outbound), of which 735 trips will be generated during the Weekday AM Peak Hour (371 inbound and 364 outbound), and 592 trips will be generated during the Weekday PM Peak Hour (304 inbound and 288 outbound). **Pass-by trips were included in the analysis. Internal Capture Rates were deducted in the analysis.**

Trips for the proposed Love’s Development are anticipated to approach and depart the Site following the distribution patterns illustrated on Figures 7.A-7.C (see pg. 30-32).

Highway Capacity Software (HCS) Version 8.1 methodology was used to analyze the current level of service at the key study intersections. *Note: According to the ODOT Analysis and Traffic Simulation Manual (OATS), Section 5.9, the operational goals for intersection analyses (TWSC and Signalized) are to operate at overall LOS D or better conditions and for each movement to operate at LOS E or better conditions.*

Under the **2043 No-Build Traffic Scenario**, the signal-controlled intersections operate at overall level of service (LOS) “C” or better conditions. In addition, the individual movements of the stop-controlled intersection operate at level of service (LOS) “D” or better conditions.

Under the **2043 Design Year Traffic Scenario Build Condition 1**, the signal-controlled intersections operate at overall level of service (LOS) “E” or better conditions. In addition, the individual movements of the stop-controlled intersections operate at level of service (LOS) “B” or better conditions with the exception of the following:

- The EBLR movement at the SR-149 & Reco Drive intersection which operates at LOS “F” during both the AM and PM Peak Hours.
- The EBLR movement at the SR-149 & Love’s Site Driveway #2 intersection which operates at LOS “F” during both the AM and PM Peak Hours.

Under the **2043 Design Year Traffic Scenario Build Condition 2**, the signal-controlled intersections operate at overall level of service (LOS) “D” or better conditions. In addition, the individual movements of the stop-controlled intersection operate at level of service (LOS) “B” or better conditions with the exception of the following:

- The EBLR movement at the SR-149 & Love’s Site Driveway #2 intersection which operates at LOS “F” during both the AM and PM Peak Hours.

The recommended improvements include the following:

- The installation of a traffic signal at the intersection of SR-149 & Reco Drive, coordinated with the signal-controlled intersections of SR-149 & I-70 WB On/Off Ramps, SR-149 & I-70 EB On/Off Ramps, and SR-149 & Bond Drive.
- Restripe the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane back to Pilot’s South Driveway.
- Construction of a NB to WB left-turn lane at the intersection of SR-149 & Love’s Site Driveway #2.
- Construction of a SB to WB right-turn lane at both intersections of SR-149 & Reco Drive and SR-149 & Love’s Site Driveway #2.
- Signal timing splits and offsets were optimized at the signal-controlled intersections.

CESO conducted turn lane analyses for the study network and reached the following conclusions:

- According to ODOT Chart 401-5b, a NB to WB left-turn lane **is warranted** at the intersections of SR-149 & Reco Drive, and SR-149 & Love’s Site Driveway #2 under the 2023 Build Traffic Scenario. According to capacity analysis results, a NB to WB left-turn lane **is warranted** at the intersection of SR-149 & I-70 WB On/Off Ramps during the 2023 Build Traffic Scenario and a SB to EB left-turn lane **is warranted** at the intersection of SR-149 & I-70 EB On/Off Ramps during the 2023 Build Traffic Scenario. However, due to the existing overpass and location of the existing bridge piers, there are physical limitations to being able to widen SR-149 and provide the additional turn lanes needed.
- According to ODOT Chart 401-6b a SB to WB right-turn lane **is warranted** at the intersections of SR-149 & Reco Drive and SR-149 & Love’s Site Driveway #2 under the 2023 Build Traffic Scenario.

CESO conducted queue length analyses for the study network and reached the following conclusions:

- The ODOT required storage length for the NBL turn lane at the SR-149 & Reco Drive intersection is 525’ of storage plus a 50’ taper. However, CESO recommends restriping the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane.
- The ODOT required storage length for the SBR turn lane at the SR-149 & Reco Drive intersection is 550’ of storage plus a 50’ taper. However, due to close proximity to the existing I-70 EB Off Ramp, CESO recommends constructing the turn lane to accommodate 200’ of storage plus a 50’ taper.
- The ODOT required storage length for the NBL turn lane at the SR-149 & Love’s Site Driveway #2 intersection is 175’ of storage plus a 50’ taper. CESO recommends constructing the turn lane to accommodate 175’ of storage plus a 50’ taper.
- The ODOT required storage length for the SBR turn lane at the SR-149 & Love’s Site Driveway #2 intersection is 175’ of storage plus a 50’ taper. CESO recommends constructing the turn lane to accommodate 175’ of storage plus a 50’ taper.

CESO conducted a traffic signal warrant study for the intersection of SR-149 & Reco Drive and reached the following conclusions:

SR-149 & Reco Drive:

- Warrant 1 – Eight-Hour Vehicular Volume **is satisfied** at the intersection under the 2023 Build and 2043 Design Year Traffic Scenarios.
- Warrant 2 – Four-Hour Vehicular Volume **is satisfied** at the intersection under the 2023 Build and 2043 Design Year Traffic Scenarios.
- Warrant 3 – Peak Hour Vehicular Volume **is satisfied** at the intersection under the 2043 Design Year Traffic Scenarios.
- Based on Warrants 1, 2, and 3 being satisfied CESO recommends the installation of a traffic signal.

1.3. Summary of Recommendations

The following summary of recommendations was generated based upon the findings in the Traffic Impact Study.

2023 No-Build Traffic Scenario (Responsibility – Others):

- No improvements are recommended or required.

2023 Build Traffic Scenario (Responsibility – Love’s):

SR-149 and Reco Drive:

- Signalize the intersection with an 80 second cycle length during the AM Peak Hour and 70 second cycle length during the PM Peak Hour. Coordinate this signal with the existing signals at the SR-149 & I-70 WB On/Off Ramps, SR-149 & I-70 EB On/Off Ramps, and SR-149 & Bond Drive intersections.
- Restripe the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane back to Pilot’s South Driveway.
- Construct a SBR turn lane to provide 200’ of storage plus a 50’ taper.

Reco Drive and Love’s Site Driveway #1

- Construct Love’s Site Driveway #1 to permit left-in, left-out, right-in, and right-out (full access) movements of passenger cars. Control this driveway with one (1) stop sign.

SR-149 and Love’s Site Driveway #2

- Construct Love’s Site Driveway #2 to permit left-in, left-out, right-in, and right-out (full access) movements of trucks and RVs. Control this driveway with one (1) stop sign.
- Construct a NBL turn lane to provide 175’ of storage plus a 50’ taper.
- Construct a SBR turn lane to provide 175’ of storage plus a 50’ taper.

2043 No-Build Traffic Scenario (Responsibility – Others):

- No improvements are recommended or required.

2043 Design Year Traffic Scenario (Responsibility – Love’s):

- No improvements are recommended or required.

Figure 12 illustrates the Love’s Development recommended improvements.

2. Introduction

This report is submitted on behalf of Love’s Travel Stops in connection with its application to Union Township, Ohio (OH) for Site Plan approval. The Traffic Impact Study (TIS) conducted by CESO, Inc. addresses the traffic related impacts associated with the proposed Love’s Travel Stop; referred to herein as “Love’s Development.”

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Love’s Site Plan application requests approval of the following access points:

- Full access auto driveway connection to Reco Drive (referred to as “Love’s Site Driveway #1”) approximately 340’ west of the SR-149 & Reco Drive intersection (centerline-to-centerline).
- Full access RV and truck driveway connection to SR-149 (referred to as “Love’s Site Driveway #2”) approximately 670’ south of Reco Drive (centerline-to-centerline).

In addition, the Love’s Site Plan application also requests approval to conduct work within the right-of-way to construct the proposed access driveways along with the recommended roadway improvements. SR-149 is under jurisdiction of the Ohio Department of Transportation (ODOT, District 11), I-70 is under jurisdiction of the Ohio Department of Transportation (ODOT, District 11), and Reco Drive is under jurisdiction of Belmont County.

This report presents the methodologies, analyses, and results of the Traffic Impact Study (TIS) for traffic generated by the proposed Love’s Development. The purpose of the TIS was to identify the traffic related impacts, if any, during typical weekday AM and PM peak hours of the adjacent street traffic corresponding with the weekday hours of operation for the proposed Love’s Development. The study parameters of this report were generated based upon a recent concept site plan, a Memorandum of Understanding (MOU) dated August 26th, 2022, between CESO and the Ohio Department of Transportation (District 11) outlining the TIS scope of services (see Appendix A).

The following intersections were analyzed in the Traffic Impact Study:

- SR-149 & I-70 WB On/Off Ramps (Signal Controlled).
- SR-149 & I-70 EB On/Off Ramps (Signal Controlled).
- SR-149 & Reco Drive (Stop Sign Controlled).

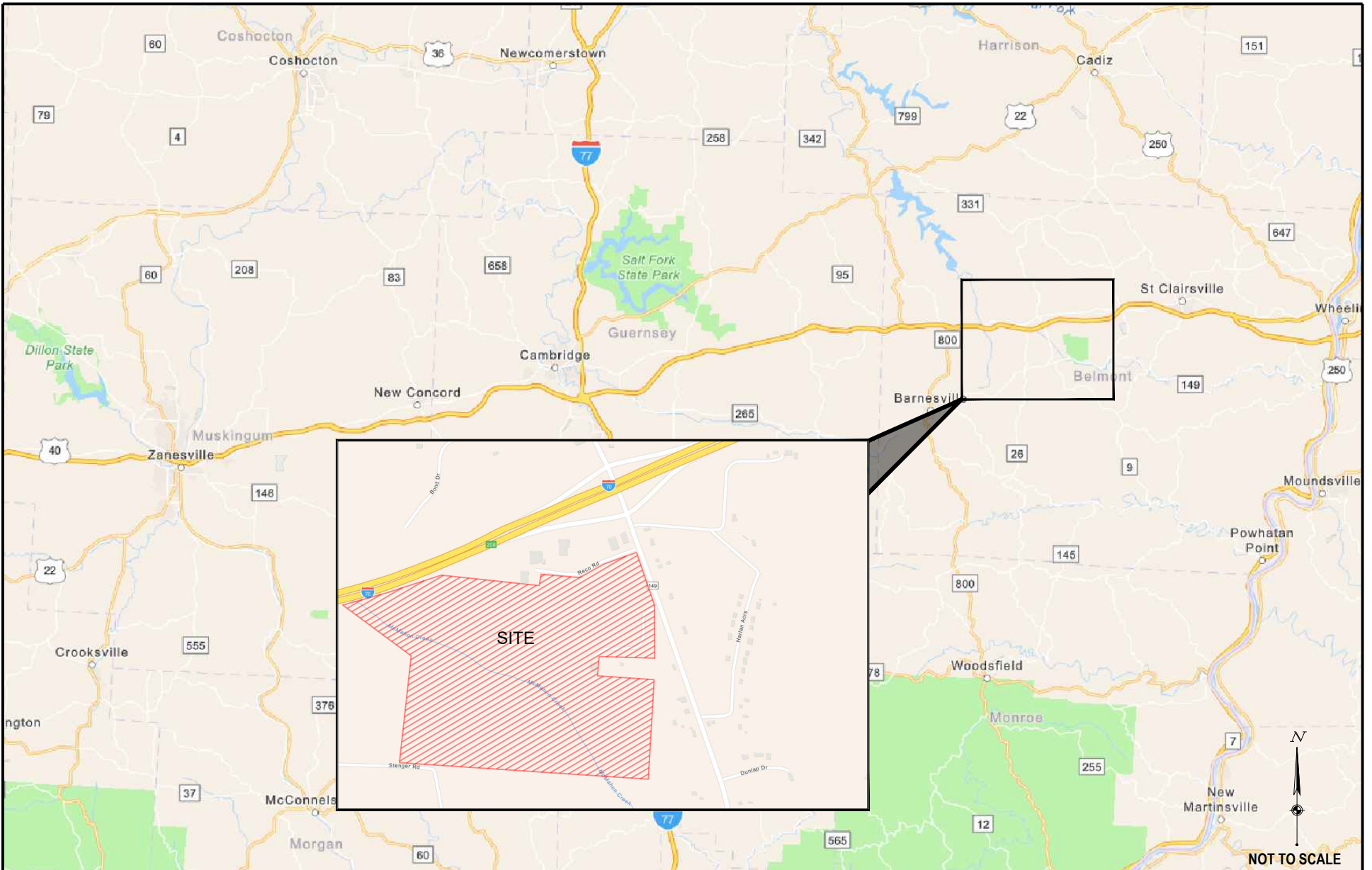
The following traffic scenarios were included in the analysis:

2043 No-Build Traffic Scenario – Represents traffic conditions during the weekday AM and PM Peak Hours of the adjacent roadway network that would exist during year 2043, without the proposed Love’s Development.

2043 Design Year Traffic Scenario Build Condition 1 – Represents traffic conditions during the weekday AM and PM Peak Hours of the adjacent roadway network that would exist during year 2043, with the proposed Love’s Development. This analysis includes the existing roadway geometry (i.e. number/types of lanes) and existing traffic control (i.e. signalized/unsignalized).

2043 Design Year Traffic Scenario Build Condition 2 – Represents traffic conditions during the weekday AM and PM Peak Hours of the adjacent roadway network that would exist during year 2043, with the proposed Love’s Development. This analysis includes the proposed roadway geometry (i.e. number/types of lanes) and proposed traffic control (i.e. signalized/unsignalized).

Figure 1 illustrates the Site location with respect to the study area and Figure 2 illustrates the Site Plan for the proposed Love’s Development.

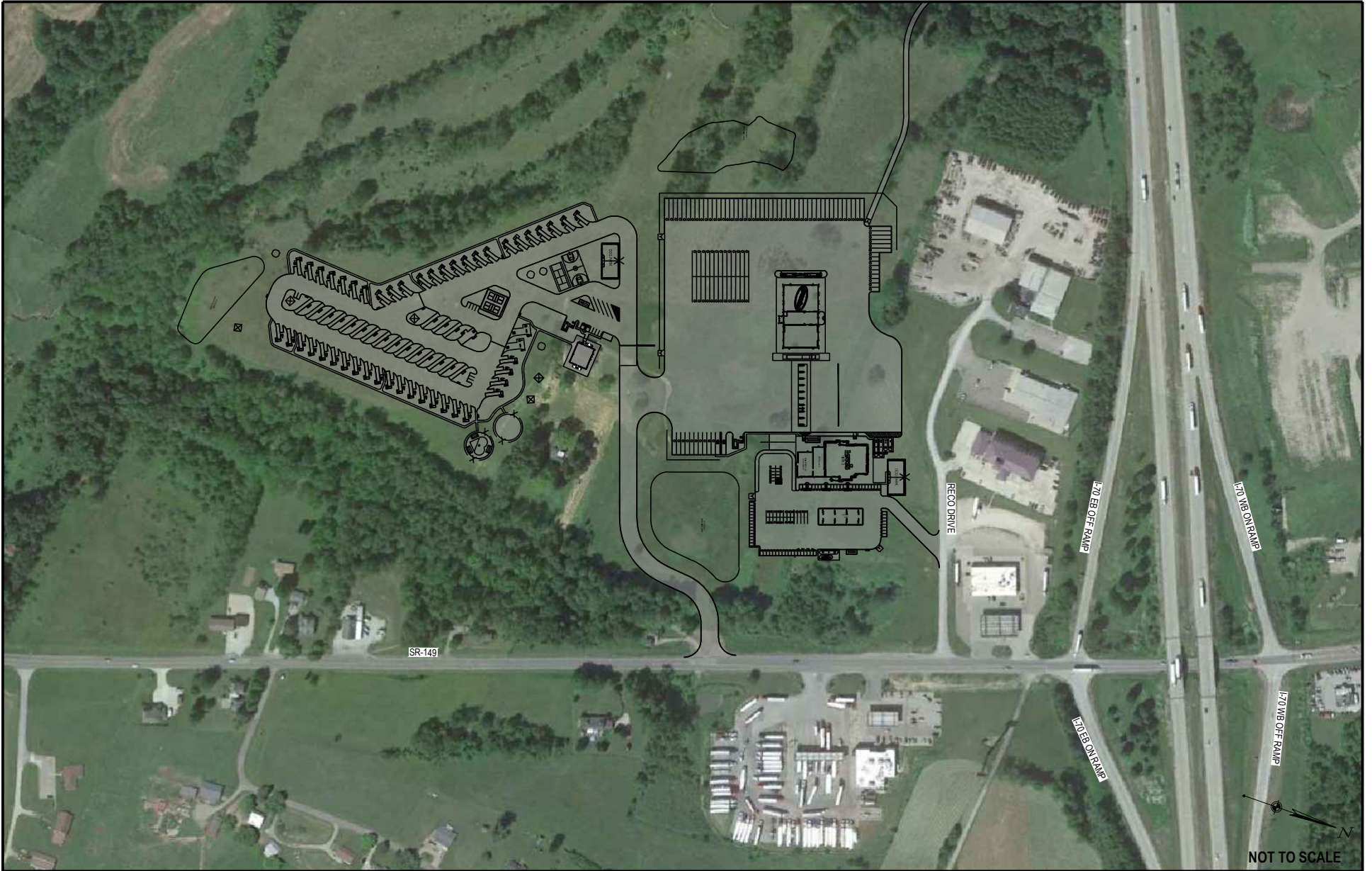


SITE LOCATION

LOVE'S DEVELOPMENT

FIGURE 1

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NOT TO SCALE

SITE PLAN

LOVE'S DEVELOPMENT

FIGURE 2

DATE:	11/21/2022
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2.1. Study Procedure

The following studies and analyses were undertaken:

1. Traffic turning movement counts were conducted by Gewalt Hamilton Associates, Inc. (GHA) on Tuesday, September 13th, 2022, between the morning hours of 7:00 AM to 9:00 AM and evening hours of 4:00 PM to 6:00 PM at the following intersections:

- SR-149 & I-70 WB On/Off Ramps (Signal Controlled).
- SR-149 & I-70 EB On/Off Ramps (Signal Controlled).

In addition, 10-hour traffic counts were conducted between 6:00 AM to 9:00 AM and 11:00 AM to 6:00 PM at the following intersection:

- SR-149 & Reco Drive (Stop Sign Controlled).

2. The Existing Traffic Volumes (Year 2022) – Cars and Trucks (Figure 4.A) were reviewed and balanced. The car and truck volumes listed in Figure 4.A were combined to form the Existing Traffic Volumes (Year 2022) – Total Volumes (Figure 4.B) for simplicity.
3. Based on Hourly Count Method 1 from the *State Highway Access Management Manual (SHAMM)*, the Existing Traffic Volumes (Year 2022) were converted to Design Hour Volumes (DHV’s). The Design Hour Volumes were calculated using the following factors determined from the Peak Hour to Design Hour Factor tables from the Ohio Department of Transportation, Modeling and Forecasting Section:

I-70 EB/WB On/Off Ramps: 1.45

SR 149: 1.13

Refer to Appendix C for Peak Hour to Design Hour Factors table used provided from Section 7.21.c of the *State Highway Access Management Manual (SHAMM)*. The 2022 Design Hour Traffic Volumes – Cars and Trucks (Figure 5.A) were combined to form the 2022 Design Hour Traffic Volumes – Total Volumes (Figure 5.B) for simplicity.

4. The 2023 No-Build Traffic Volumes – Cars and Trucks (Figure 6.A) were calculated by applying growth rates to the 2022 Design Hour Traffic Volumes (Figure 5.A). The growth rates were calculated using the linear regression method referenced in the *Ohio Traffic Forecasting Manual, Volume 2, Section 4.2* for all study roadways. A linear growth rate of 0.5 percent (%) was applied for one (1) year to the study roadways (growth factor of 1.005). The car and truck volumes shown on Figure 6.A were combined to form the 2023 No-Build Traffic Volumes – Total Volumes (Figure 6.B) for simplicity. The growth rate documentation is included in Appendix C of the report.
5. Directional distribution analyses were conducted to determine the potential distribution of patrons for the proposed Love’s Development under the 2023 Build and 2043 Design Year Traffic Scenarios (see Figures 7.A-7.C).
6. Analyses were conducted to determine the potential traffic volumes generated by the proposed Love’s Development under the 2023 Build and 2043 Design Year Traffic Scenarios utilizing data provided in the Institute of Transportation Engineers’ *Trip Generation Manual, 11th Edition* (see Table 1).

7. Addition of the Site Generated Traffic Volumes (Figures 8.A-8.C) to the 2023 and 2043 No-Build Traffic Volumes (Figures 6.A-6.B and Figures 10.A-10.B) to reflect the 2023 Build and 2043 Design Year Traffic Volumes (Figures 9.A-9.B and Figures 11.A-11.B).
8. The 2043 No-Build Traffic Volumes – Cars and Trucks (Figure 10.A) were calculated by applying growth rates to the 2022 Design Hour Traffic Volumes (Figure 5.A). The growth rates were calculated using the linear regression method referenced in the *Ohio Traffic Forecasting Manual, Volume 2, Section 4.2* for all study roadways. A linear growth rate of 0.5 percent (%) was applied for twenty-one (21) years to the study roadways (growth factor of 1.105). The car and truck volumes shown on Figure 10.A were combined to form the 2043 No-Build Traffic Volumes – Total Volumes (Figure 10.B) for simplicity. The growth rate documentation is included in Appendix C of the report.
9. Capacity analyses of the 2043 No-Build Traffic Volumes (Figures 10.A-10.B) were completed to determine the capacity of the key study intersections during AM and PM Peak Hours using Highway Capacity Software (HCS) Version 8.1 methodology.
10. Capacity analyses of the 2043 Design Year Traffic Volumes Build Condition 1 (Figures 11.A-11.B) were completed to determine the capacity of the key study intersections during AM and PM Peak Hours using HCS Version 8.1 methodology.
11. Capacity analyses of the 2043 Design Year Traffic Volumes Build Condition 2 (Figures 11.A-11.B) were completed to determine the capacity of the key study intersections (with improvements) during AM and PM Peak Hours using HCS Version 8.1 methodology.
12. Turn lane analyses were completed to determine if left-turn lanes or right-turn lanes were required as a result of the Love’s Development. Turn-lane analyses utilized ODOT charts for unsignalized free-flowing approaches and capacity analyses results for signal-controlled intersections.
13. Queue Length Analyses were completed using the 95th percentile queue lengths provided from the HCS capacity results.
14. CESO conducted a Signal Warrant Study at the intersection of SR-149 and Reco Drive.
15. Recommendations for roadway improvements were generated under the 2043 Design Year Condition 2 Traffic Scenario based upon the capacity/queue length analyses of the surrounding roadway network. Application of the recommendations and evaluation of the capacity at the key study intersections under the applicable traffic scenarios, during AM and PM Peak Hours, were completed using HCS Version 8.1 methodology.

2.2. References

This report utilizes information provided by the following sources:

1. *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis*. Transportation Research Board, Washington, D.C., 2016.
2. *Trip Generation Manual*. 11th ed. Washington, DC: Institute of Transportation Engineers, 2022.
3. Most recent Site Plan obtained from Love’s.
4. *Ohio Manual of Uniform Traffic Control Devices (OMUTCD)*. Ohio Department of Transportation (ODOT).
5. “St. Clairsville, OH.” 40°03’20.60” N and 81°03’15.36” W, *Google Earth*. June 3rd, 2022.
6. *Location & Design Manual – Volume I (July 2022)*. Ohio Department of Transportation (ODOT).
7. *State Highway Access Management Manual (July 2021)*. Ohio Department of Transportation – Office of Roadway Engineering.
8. *ODOT Analysis and Traffic Simulation Manual (OATS, July 2021)*. Ohio Department of Transportation (ODOT).

3. Roadway and Traffic Conditions in the Vicinity of the Site

An inventory of existing transportation conditions in the vicinity of the Site was created to form a database for use in projecting Build conditions.

3.1. Study Location and Area Land Use

The proposed Love’s Development is to be located in the southwest corner of SR-149 and Reco Drive, within Union Township, Belmont County, OH. Land use in the direct vicinity of the Site is primarily commercial, residential, and agricultural.

Access to the proposed Love’s Development is projected via two (2) access points on the adjacent roadway; one full access auto driveway connection to Reco Drive (referred to as “Love’s Site Driveway #1”) approximately 340’ west of SR-149 (centerline-to-centerline) and one full access RV and truck driveway connection to SR-149 (referred to as “Love’s Site Driveway #2”) approximately 670’ south of Reco Drive (centerline-to-centerline).

3.2. Area Roadway Characteristics

SR-149 – SR-149 runs in the north/southbound direction in the vicinity of the Site. SR-149 is a two-way roadway with one lane in each direction. SR-149 is classified as a Major Collector according to the *ODOT Transportation Information Mapping System*. SR-149 is under jurisdiction of the Ohio Department of Transportation (ODOT, District 11). The posted speed limit on SR-149 is 45 mph.

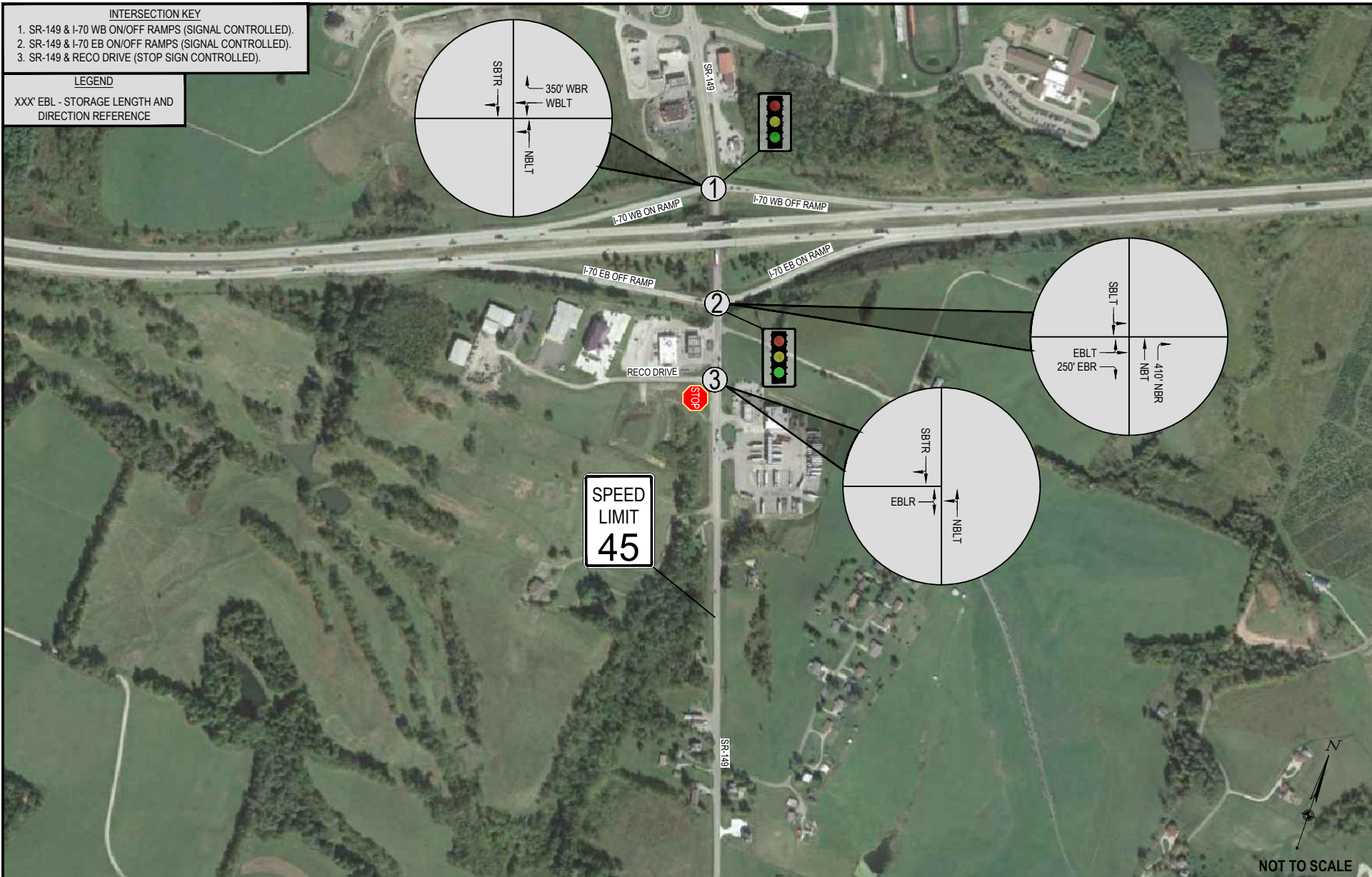
I-70 On/Off Ramps – I-70 runs in the east/westbound direction in the vicinity of the Site. I-70 is a two-lane divided highway. I-70 is classified as a Principal Arterial Interstate according to the *ODOT Transportation Information Mapping System*. I-70 is under jurisdiction of the Ohio Department of Transportation (ODOT, District 11). The speed limit on I-70 is unposted and assumed to be 70 mph.

Reco Drive – Reco Drive runs in the east/westbound direction in the vicinity of the Site. Reco Drive is a two-way roadway with one lane in each direction. Reco Drive is classified as a Local Road according to the *ODOT Transportation Information Mapping System*. Reco Drive is under jurisdiction of Belmont County. The speed limit on Reco Drive is unposted and assumed to be 25 mph.

The Existing Transportation System is shown on Figure 3 of the report.

INTERSECTION KEY
 1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
 2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
 3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

LEGEND
 XXX' EBL - STORAGE LENGTH AND
 DIRECTION REFERENCE



3.3. Existing Traffic Volumes

Traffic turning movement counts were conducted by Gewalt Hamilton Associates, Inc. (GHA) on Tuesday, September 13th, 2022, between the morning hours of 7:00 AM to 9:00 AM and evening hours of 4:00 PM to 6:00 PM at the following intersections:

- SR-149 & I-70 WB On/Off Ramps (Signal Controlled).
- SR-149 & I-70 EB On/Off Ramp (Signal Controlled).

In addition, 10-hour traffic counts were conducted between 6:00 AM to 9:00 AM and 11:00 AM to 6:00 PM at the following intersection:

- SR-149 & Reco Drive (Stop Sign Controlled).

The weekday peak hours of the Traffic Impact Study roadway network were determined to occur between the hours of:

- 7:00 AM – 8:00 AM (AM Peak Hour).
- 4:30 PM – 5:30 PM (PM Peak Hour).

Count data collected consists of turning movement counts with classification breakouts of lights, buses, single-unit trucks, and articulated trucks.

The Existing Traffic Count Data and Signal Timing Sheets are located in Appendix B of the report. The Existing Weekday Peak Hour Traffic Volumes (Year 2021) are illustrated on Figure 4.A (Cars and Trucks) and Figure 4.B (Total Volumes).

INTERSECTION KEY

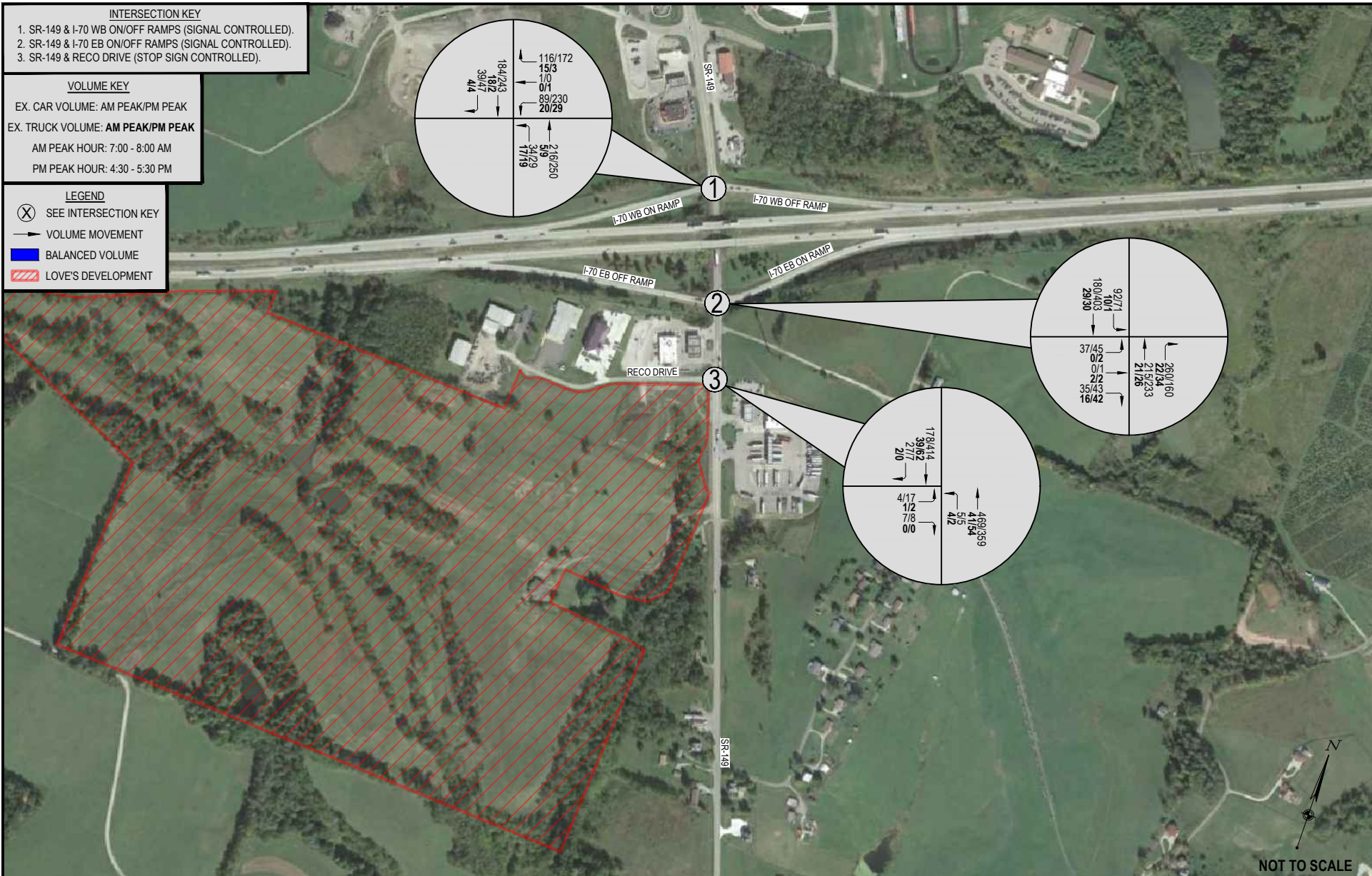
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

EX. CAR VOLUME: AM PEAK/PM PEAK
 EX. TRUCK VOLUME: **AM PEAK/PM PEAK**
 AM PEAK HOUR: 7:00 - 8:00 AM
 PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

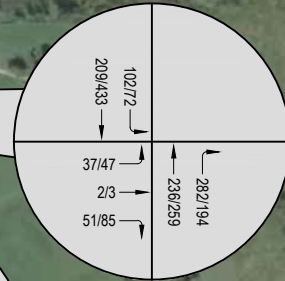
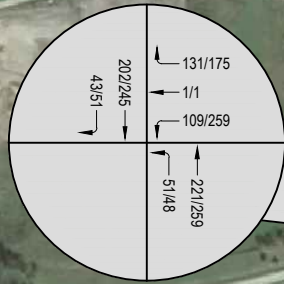
EX. TOTAL VOLUME: AM PEAK/PM PEAK

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

EXISTING TRAFFIC VOLUMES (YEAR 2022) - TOTAL VOLUMES

UNION TOWNSHIP		LOVE'S DEVELOPMENT		BELMONT, OHIO	
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FIGURE 4.B	
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3.4. Design Hour Volumes

According to the *Ohio Certified Traffic Manual*, “Design Hour Volumes are the number of vehicles that travel through a segment of roadway during the design hour. The DHV is the volume unit which is used for making roadway structural and capacity design decisions”. Traffic volumes vary considerably during different hours of the day. Design Hour Volumes include applying a design hour factor to existing volumes.

Based on the Hourly Count Method 1 from the ODOT *State Highway Access Management Manual*, the Existing Weekday Peak Hour Traffic Volumes (Year 2022) – Cars and Trucks (Figure 4.A) were multiplied by a DHV factor of 1.45 to the I-70 EB and WB On/Off Ramps, and 1.13 to SR-149 volumes to obtain the 2022 Design Hour Weekday Peak Hour Traffic Volumes – Cars and Trucks (Figure 5.A). The DHV factors were acquired from the Office of Statewide Planning & Research’s website, Modeling & Forecasting section. Refer to Appendix C of the report for further documentation.

For simplicity, the traffic volumes listed on Figure 5.A were combined to form the 2022 Design Hour Weekday Peak Hour Traffic Volumes – Total Volumes shown on Figure 5.B.

INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

2021 DHV CAR VOLUME: AM PEAK/PM PEAK

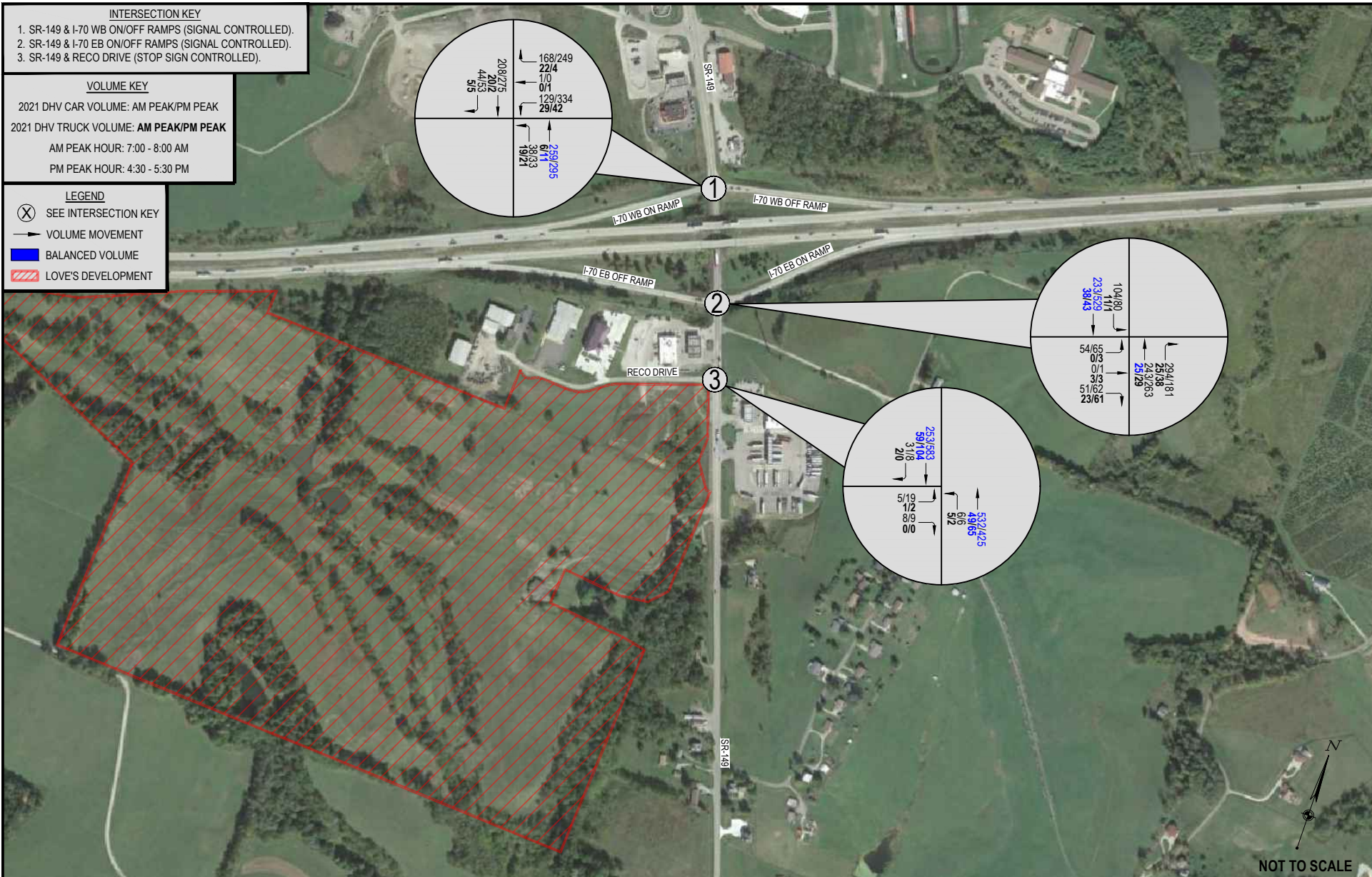
2021 DHV TRUCK VOLUME: **AM PEAK/PM PEAK**

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

2022 DHV TOTAL VOLUME: AM PEAK/PM PEAK

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

4. Estimates of 2023 No-Build Traffic in the Vicinity of the Site

4.1. 2023 No-Build Traffic Volumes

The 2023 No-Build Traffic Volumes – Cars and Trucks (Figure 6.A) were calculated by applying growth rates to the 2022 Design Hour Traffic Volumes (Figure 5.A). The growth rates were calculated using the linear regression method referenced in the *Ohio Traffic Forecasting Manual, Volume 2, Section 4.2* for all study roadways. A linear growth rate of 0.5 percent (%) was applied for one (1) year to the study roadways (growth factor of 1.005). The car and truck volumes shown on Figure 6.A were combined to form the 2023 No-Build Traffic Volumes – Total Volumes (Figure 6.B) for simplicity. The growth rate documentation is included in Appendix C of the report.

INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

2023 NO-BUILD CAR VOLUME: AM PEAK/PM PEAK

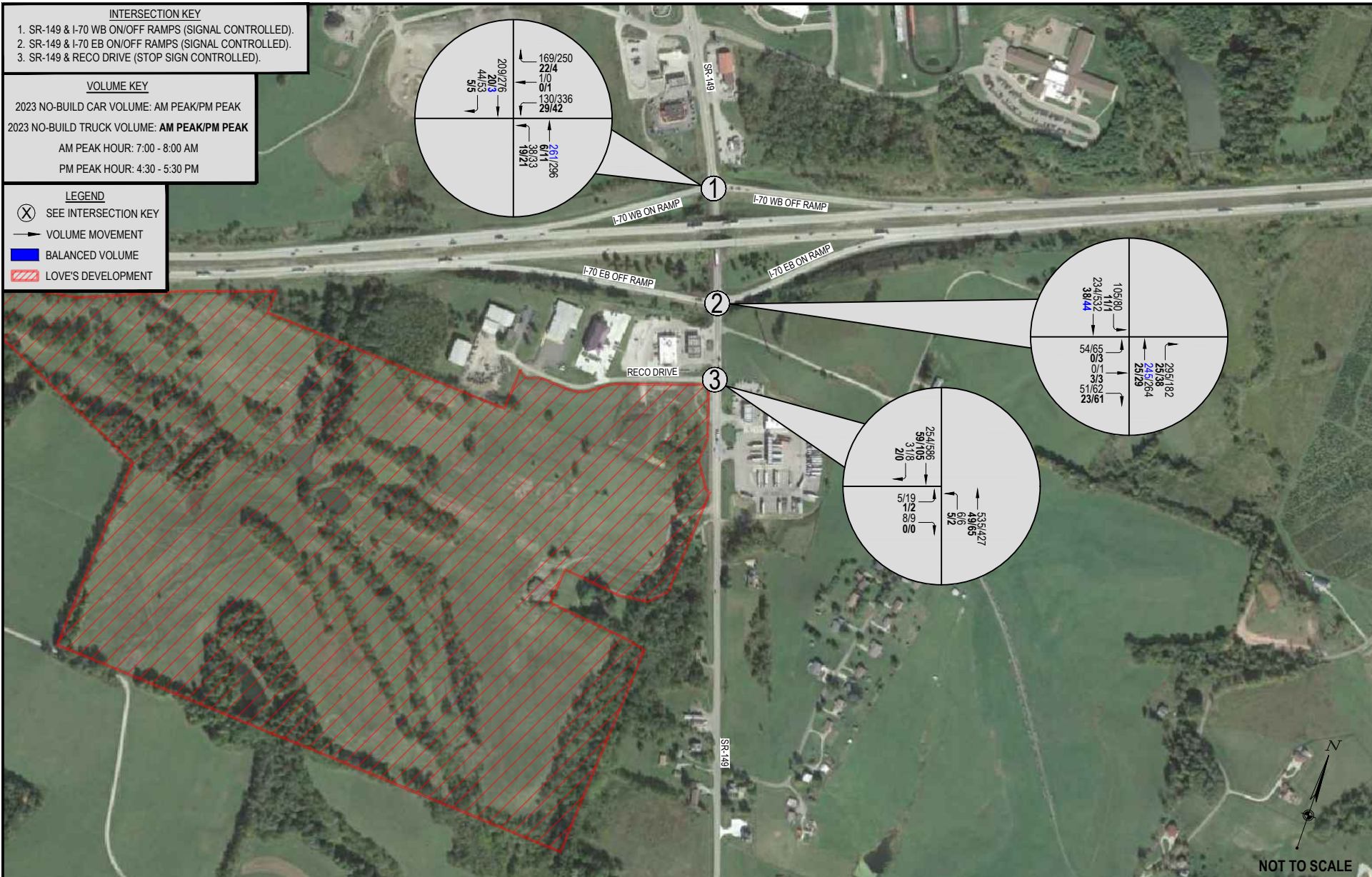
2023 NO-BUILD TRUCK VOLUME: **AM PEAK/PM PEAK**

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

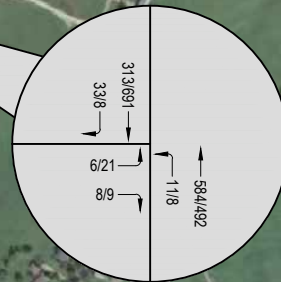
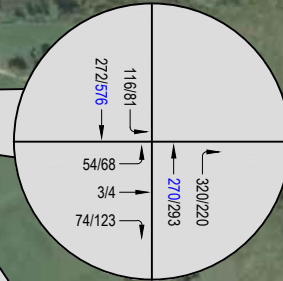
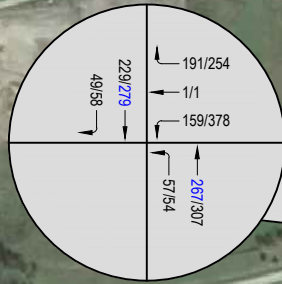
2023 NO-BUILD TOTAL VOLUME: AM PEAK/PM PEAK

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

2023 NO-BUILD TRAFFIC VOLUMES - TOTAL VOLUMES

LOVE'S DEVELOPMENT

FIGURE 6.B

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5. Trip Generation

5.1. Site Generated Traffic Volumes

Studies of similar developments throughout North America have shown that the amount of traffic generated will be functionally related to some unit of activity (i.e., number of fueling stations, gross floor area, service bays, etc.). In development, site traffic fluctuates substantially on different days and hours throughout the year. Therefore, it is imperative to select an appropriate hourly volume on which to base the design of the external roadway and site access facilities. The Weekday AM and PM Peak Hours were selected based on the adjacent street traffic during this hour.

The 2023 Build and 2043 Design Year Traffic Scenarios include the proposed use of the Site as a Love’s Development that consists of:

- Gasoline Service Station with a 11,300 S.F. Convenience Market consisting of 25 fueling positions (16 passenger car and 9 truck).
- Fast-Food Restaurant with Drive-through Window occupying approximately 3,100 S.F.
- Fast-Food Restaurant without Drive-through Window occupying approximately 2,900 S.F.
- Tire Store with 3 service bays.
- Campground/Recreational Vehicle Park with 70 campsites.

In addition, the Love’s Development is projected to have approximately 104 parking spaces for passenger cars, 97 parking spaces for semi-trucks and 70 total RV parking spaces.

For analysis purposes, the base variable units for the trip-generation rates were fueling positions, KSF (Ex. 3.24 KSF = 3,240 S.F.), service-bays, and campsites. The Site Generated Traffic Volumes (Table 1) were calculated by utilizing data contained in the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*, in combination with methods outlined in the (ITE) Trip Generation Handbook.

Internal Capture Rates were calculated using the methodology contained in the (ITE) *Trip Generation Handbook, 3rd Edition* and using the NCHRP 684 Internal Trip Capture Estimation Tool spreadsheet. For the AM Peak Hour, the internal capture rate was calculated as 13%; for the PM Peak Hour, the internal capture rate was calculated as 19%. The estimation tool also provided internal capture percentages for the entering and exiting traffic based on land use and peak hour. The corresponding internal capture percentage was applied to each land use. In addition, pass-by trips were applied and based on percentages found in the (ITE) *Trip Generation Manual, 11th Edition*. The NCHRP Summary Sheets and ITE Land Use Calculations are located in Appendix D of the report.

The Site Generated Traffic Volumes are presented below in Table 1.

Table 1
Site Generated Traffic Volumes

ITE Land Use Description	ITE Cat.	Size	Unit	Total Generated Trips										
				Weekday			Weekday AM Peak Hour				Weekday PM Peak Hour			
				Tot	In	Out	^A Tot	In	Out	^B PB	^A Tot	In	Out	^B PB
Passenger Car														
Gasoline/Service Station with Convenience Market	945	16	Fuel Pos.	4,114	2,057	2,057	432	52	52	328	365	46	45	274
<i>ITE Cat. 945 Entering (%) / Exiting (%)</i>				100%	50%	50%	100%	50%	50%	^C 76%	100%	50%	50%	^C 75%
Internal Capture Applied				-----	-----	-----	391	49	46	296	318	38	40	240
<i>Internal Capture Rates</i>				-----	-----	-----	-----	6%	13%	-----	-----	15%	11%	-----
Fast-Food Restaurant with Drive-Through Window	934	3,100	S.F.	1,450	725	725	138	35	33	70	102	24	22	56
<i>ITE Cat. 934 Entering (%) / Exiting (%)</i>				100%	50%	50%	100%	51%	49%	^C 50%	100%	52%	48%	^C 55%
Internal Capture Applied				-----	-----	-----	110	26	30	54	66	17	13	36
<i>Internal Capture Rates</i>				-----	-----	-----	-----	26%	14%	-----	-----	29%	41%	-----
Fast-Food Restaurant without Drive-Through Window	933	2,900	S.F.	1,306	653	653	125	73	52	0	96	48	48	0
<i>ITE Cat. 933 Entering (%) / Exiting (%)</i>				100%	50%	50%	100%	58%	42%	^C 0%	100%	50%	50%	^C 0%
Internal Capture Applied				-----	-----	-----	99	54	45	0	62	34	28	0
<i>Internal Capture Rates</i>				-----	-----	-----	-----	26%	14%	-----	-----	29%	41%	-----
Total (No Internal Capture Applied)				6,870	3,435	3,435	695	160	137	398	563	118	115	330
Total (Internal Capture Subtracted)				6,870	3,435	3,435	600	129	121	350	446	89	81	276
RV/Truck/Semi-Trailer														
Campground/Recreational Vehicle Park	416	70	Camp-sites	N/A	N/A	N/A	15	5	10	0	19	12	7	0
<i>ITE Cat. 416 Entering (%) / Exiting (%)</i>				-----	-----	-----	100%	36%	64%	^C 0%	100%	65%	35%	^C 0%
Internal Capture Applied				-----	-----	-----	15	5	10	0	19	12	7	0
<i>Internal Capture Rates</i>				-----	-----	-----	-----	N/A	N/A	-----	-----	N/A	N/A	-----
Truck Stop	950	9	Fuel Pos.	2,016	1,008	1,008	126	62	64	0	138	73	65	0
<i>ITE Cat. 945 Entering (%) / Exiting (%)</i>				100%	50%	50%	100%	49%	51%	^C 0%	100%	53%	47%	^C 0%
Internal Capture Applied				-----	-----	-----	114	58	56	0	119	62	57	0
<i>Internal Capture Rates</i>				-----	-----	-----	-----	6%	13%	-----	-----	15%	11%	-----
Tire Store	848	3	Srv. Bay	-----	-----	-----	6	4	2	0	10	4	6	0
<i>ITE Cat. 848 Entering (%) / Exiting (%)</i>				-----	-----	-----	100%	64%	36%	^C 0%	100%	42%	58%	^C 0%
Internal Capture Applied				-----	-----	-----	6	4	2	0	8	3	5	0
<i>Internal Capture Rates</i>				-----	-----	-----	-----	6%	13%	-----	-----	15%	11%	-----
Total (No Internal Capture Applied)				2,016	1,008	1,008	147	71	76	0	167	89	78	0
Total (Internal Capture Subtracted)				2,016	1,008	1,008	135	67	68	0	146	77	69	0
Net Trip Generation Summary				8,886	4,443	4,443	735	196	189	350	592	166	150	276

^A – Primary Trips + Pass-by Trips, ^B – Pass-by Trips Generated, ^C – Percent (%) of ^ATot, * - Volume rounded to even number

The full buildout of the Love’s Travel Stop is estimated to generate 8,886 trips per day on a typical weekday (4,443 inbound and 4,443 outbound), of which 735 trips will be generated during the Weekday AM Peak Hour (371 inbound and 364 outbound) and 592 trips will be generated during the Weekday PM Peak Hour (304 inbound and 288 outbound).

Appendix D includes trip generation calculations and ITE Trip Generation Category 416, 848, 933, 934, 945, and 950 Sheets utilized to calculate the values presented in Table 1.

5.2. Directional Distribution of Site Generated Traffic Volumes

The directional distribution of the development-generated traffic is a function of several variables. The assumptions and methods used in estimating the direction in which traffic will approach and depart the Site varies with several location-specific conditions such as:

- Size and type of the proposed development.
- Population distribution within the defined area of influence.
- Prevailing operating conditions on the existing street system.

The analysis of directional distribution is based on the observation that drivers normally will choose the fastest (not necessarily the most direct) routes to and from a given traffic generator.

The internal trip assignment was based upon the proposed Site Plan and the understanding of the Love’s Development operation. The traffic entering and exiting the development will not always travel the most direct route.

The anticipated directional distribution of trips generated by the proposed Love’s Development is summarized in Table 2. Additionally, Figures 7.A-7.B illustrate the primary and pass-by/diverted directional distribution for passenger cars and Figure 7.C illustrates the primary/diverted directional distribution for trucks.

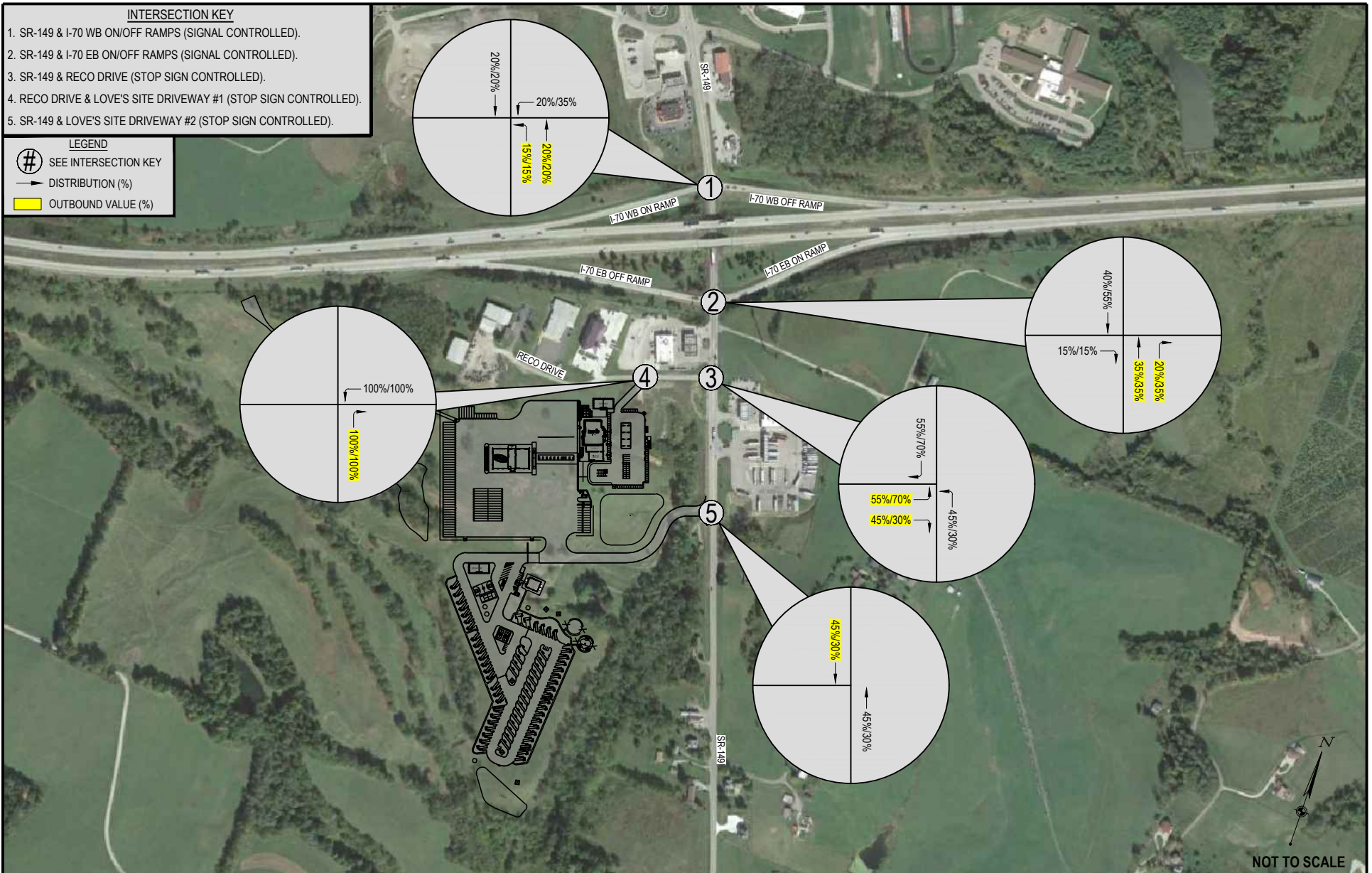
Table 2
Directional Distribution of Site Generated Traffic Volumes

Route	Distribution Approach/Departure	
	Passenger Cars	
	AM Peak Hour	PM Peak Hour
Primary Trip Distribution - Cars (Figure 7.A)		
To/From the East via I-70	20%/20%	35%/35%
To/From the West via I-70	15%/15%	15%/15%
To/From the North via SR-149	20%/20%	20%/20%
To/From the South via SR-149	45%/45%	30%/30%
TOTAL	100%/100%	100%/100%
Pass-by/Diverted Trip Distribution – Cars (Figure 7.B)		
Pass-by from the North/To the South via SR-149	25%/25%	20%/20%
Pass-by from the South/To the North via SR-149	25%/25%	15%/15%
Diverted from the East/To the West via I-70	30%/30%	45%/45%
Diverted from the West/To the East via I-70	20%/20%	20%/20%
TOTAL	100%/100%	100%/100%

Table 2 – Continued.
Directional Distribution of Site Generated Traffic Volumes

Route	Distribution Approach/Departure	
	Trucks	
	AM Peak Hour	PM Peak Hour
Primary/Diverted Trip Distribution - Trucks (Figure 7.C)		
To/From the North via SR-149	15%/15%	5%/5%
To/From the South via SR-149	25%/25%	15%/15%
Diverted from the East/To the West via I-70	40%/40%	35%/35%
Diverted from the West/To the East via I-70	20%/20%	45%/45%
TOTAL	100%/100%	100%/100%

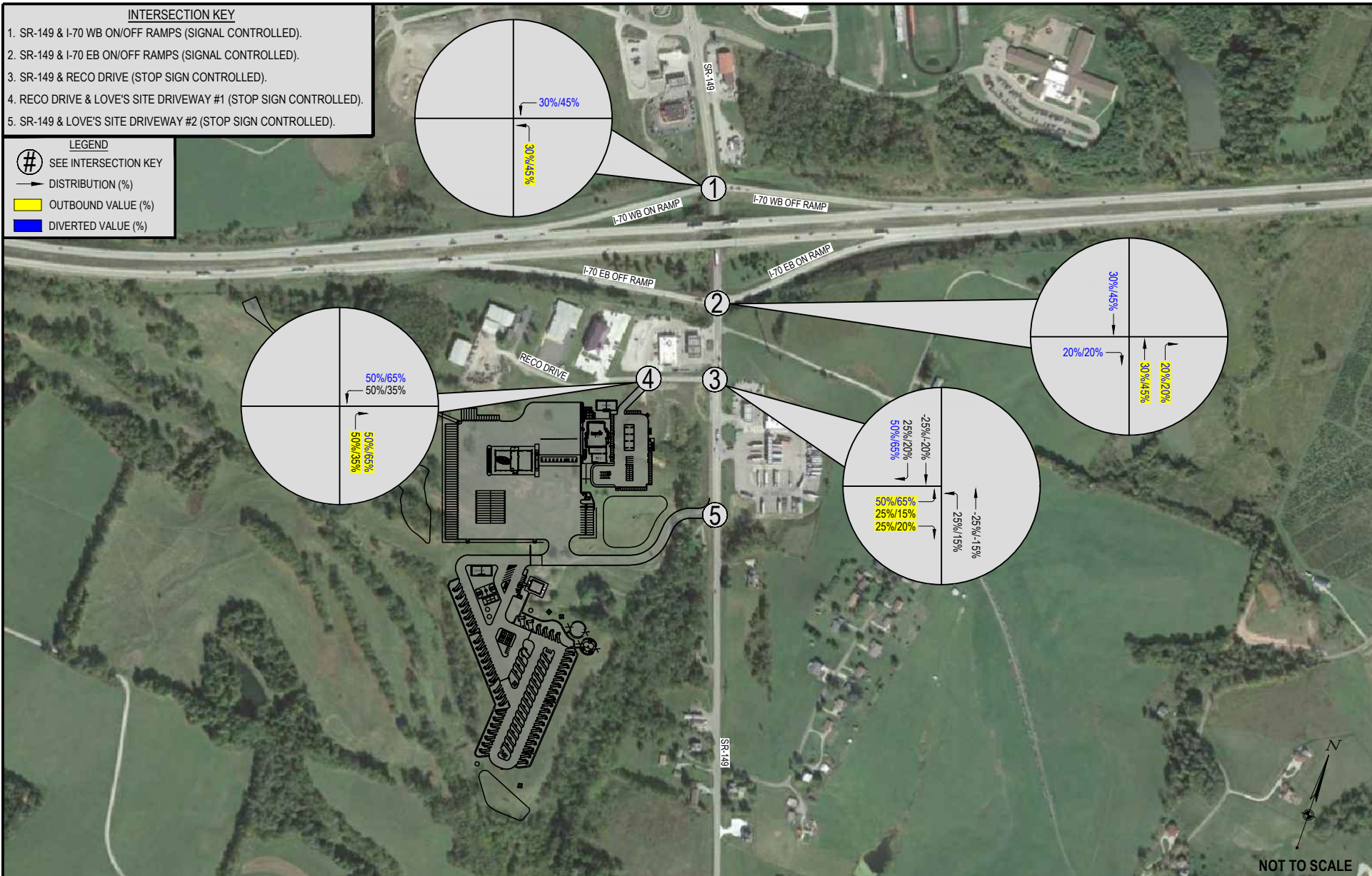
Based upon the directional distributions listed in Table 2 and illustrated on Figures 7.A-7.C, the estimated Site Generated Traffic Volumes shown in Table 1 were distributed to the adjacent roadway system. The Site Generated Traffic Volumes are illustrated on Figures 8.A-8.C.



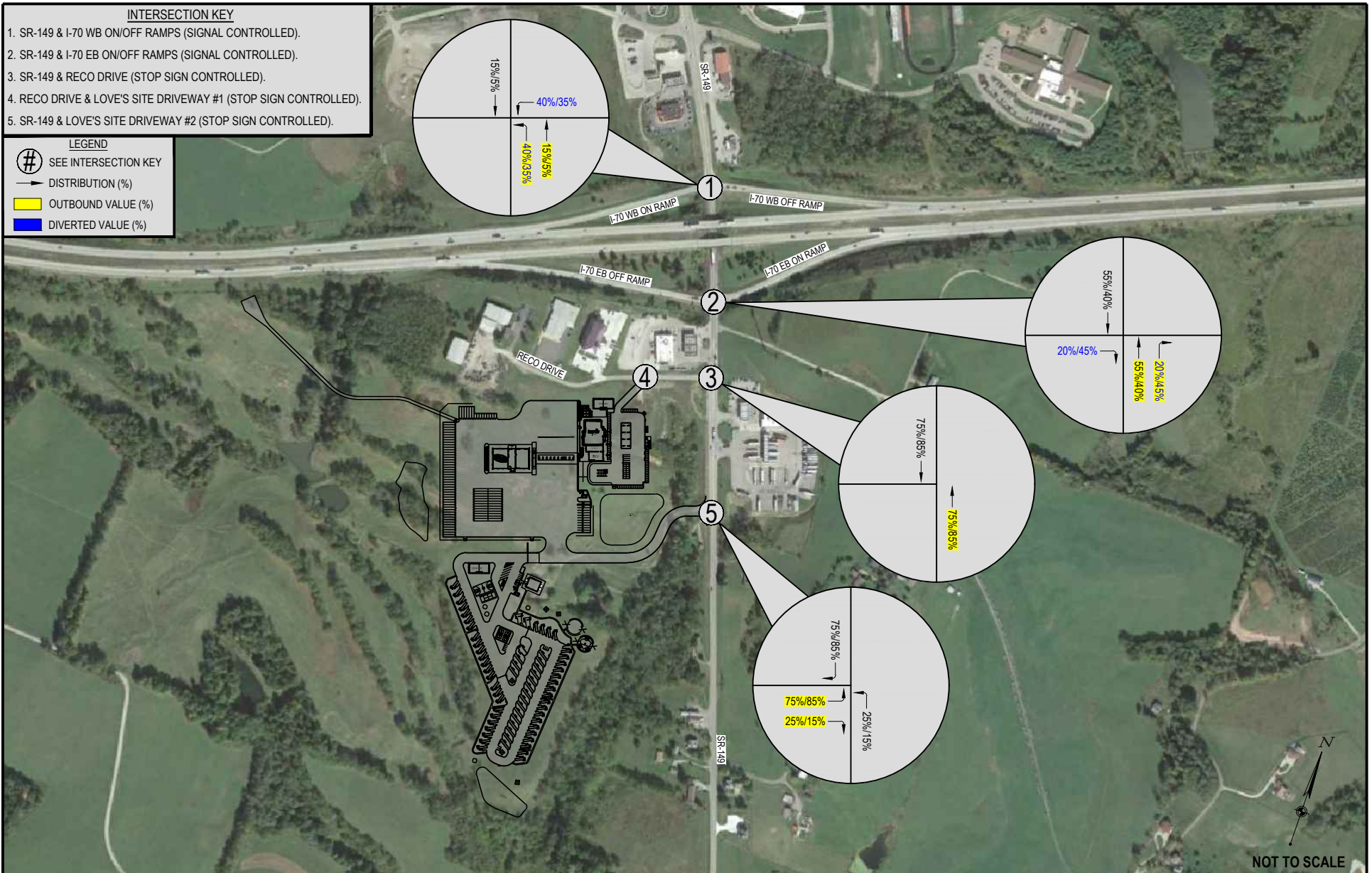
NOT TO SCALE

- INTERSECTION KEY**
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
 2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
 3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
 4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
 5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

- LEGEND**
- # SEE INTERSECTION KEY
 - DISTRIBUTION (%)
 - OUTBOUND VALUE (%)
 - DIVERTED VALUE (%)



NOT TO SCALE



NOT TO SCALE

- INTERSECTION KEY**
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
 2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
 3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
 4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
 5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

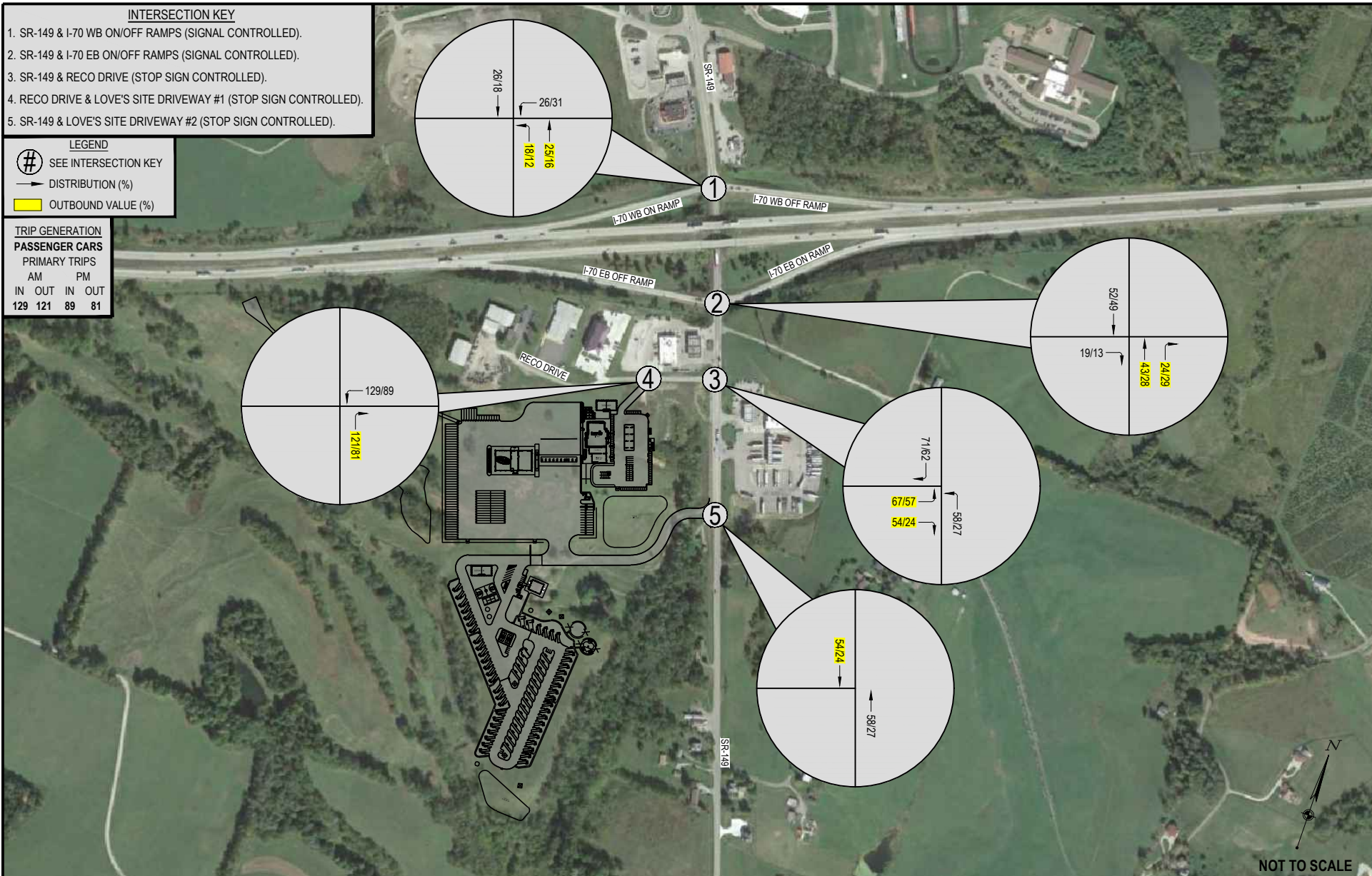
- LEGEND**
- # SEE INTERSECTION KEY
 - DISTRIBUTION (%)
 - OUTBOUND VALUE (%)

TRIP GENERATION

PASSENGER CARS

PRIMARY TRIPS

AM		PM	
IN	OUT	IN	OUT
129	121	89	81



NOT TO SCALE

- INTERSECTION KEY**
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
 2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
 3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
 4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
 5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

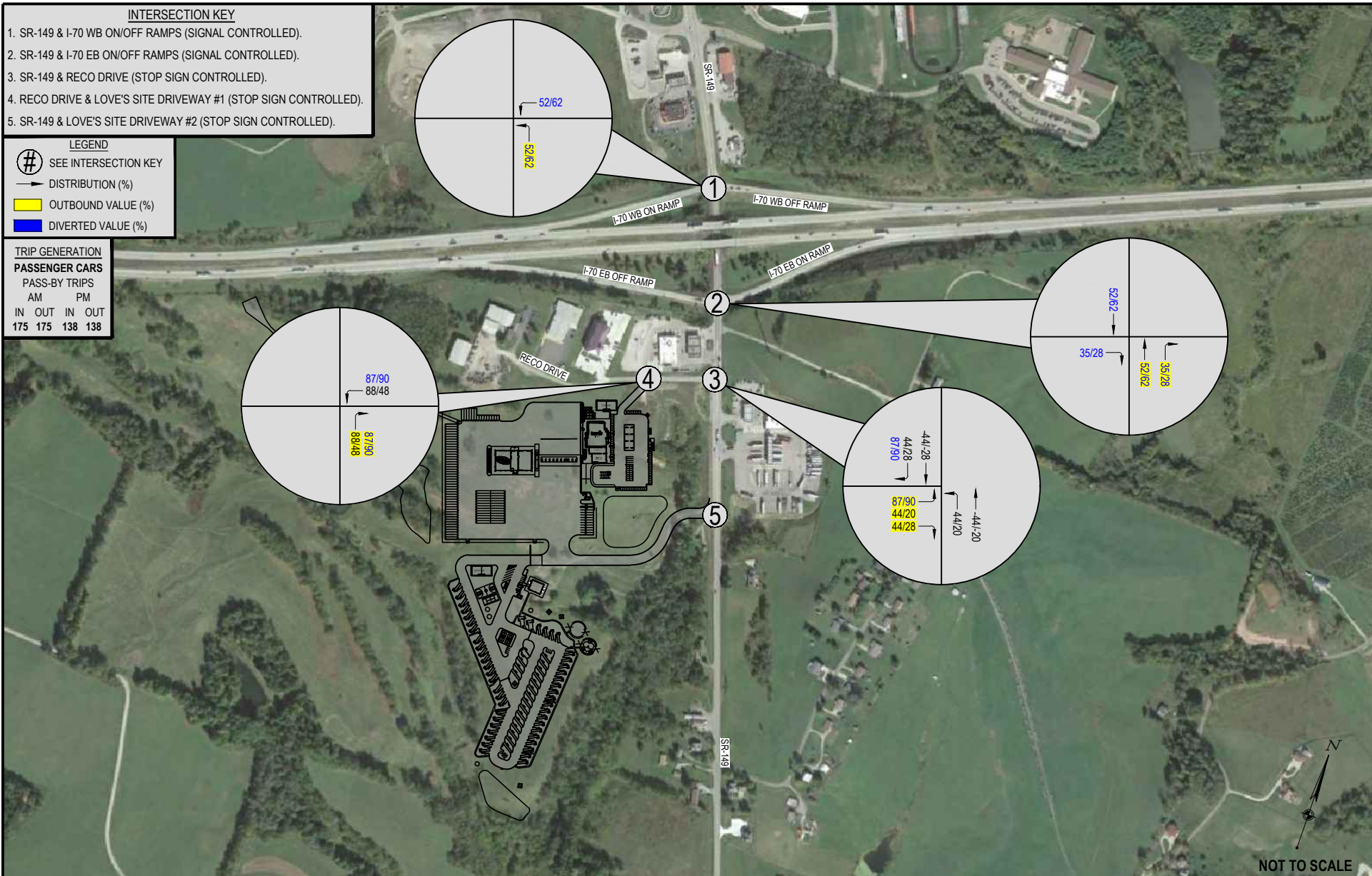
- LEGEND**
- # SEE INTERSECTION KEY
 - DISTRIBUTION (%)
 - OUTBOUND VALUE (%)
 - DIVERTED VALUE (%)

TRIP GENERATION

PASSENGER CARS

PASS-BY TRIPS

	AM	PM
IN	175	138
OUT	175	138



NOT TO SCALE

- INTERSECTION KEY**
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
 2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
 3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
 4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
 5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

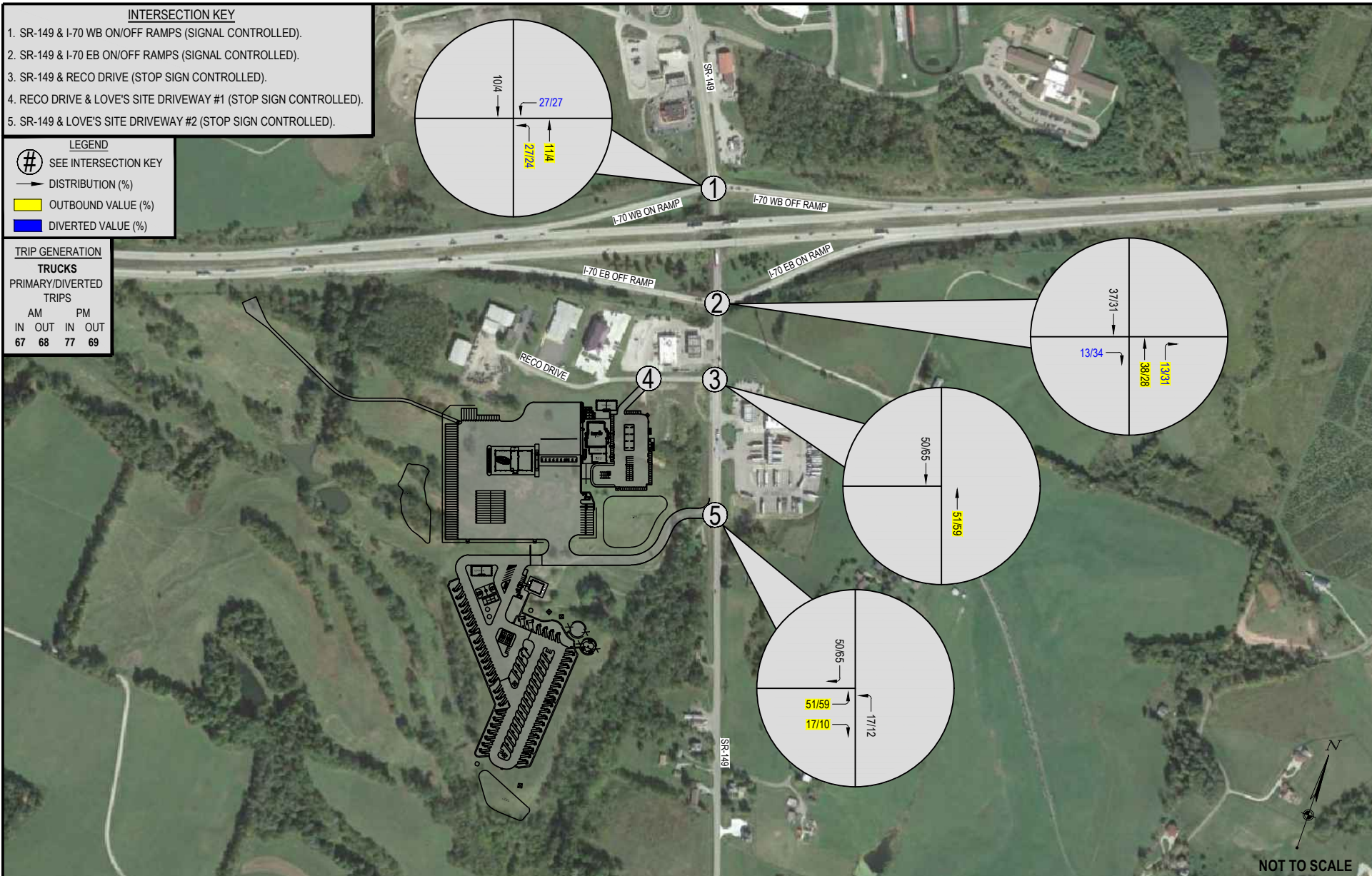
- LEGEND**
- # SEE INTERSECTION KEY
 - DISTRIBUTION (%)
 - OUTBOUND VALUE (%)
 - DIVERTED VALUE (%)

TRIP GENERATION

TRUCKS

PRIMARY/DIVERTED TRIPS

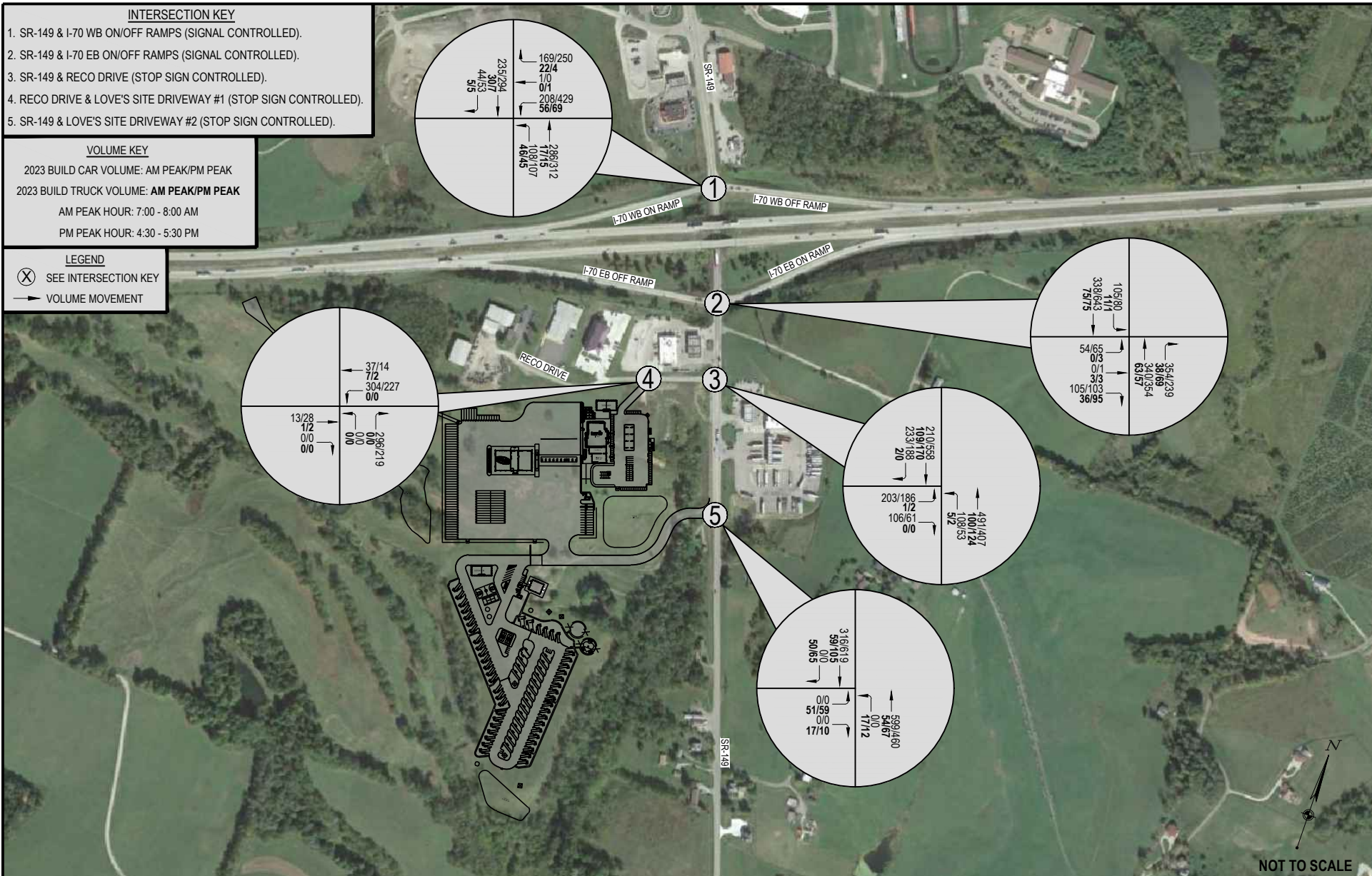
	AM	PM
IN	67	68
OUT	77	69



6. Estimates of 2023 Build Traffic in the Vicinity of the Site

6.1. 2023 Build Traffic Volumes

The 2023 Build Traffic Volumes (Figures 9.A-9.B) were calculated by adding the Site Generated Traffic Volumes (Figures 8.A-8.C) to the 2023 No-Build Traffic Volumes – Cars and Trucks (Figure 6.A). The car and truck volumes shown on Figure 9.A were combined to form the 2023 Build Traffic Volumes – Total Volumes (Figure 9.B) for simplicity.



NOT TO SCALE

INTERSECTION KEY

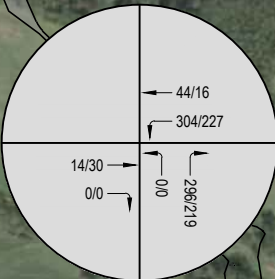
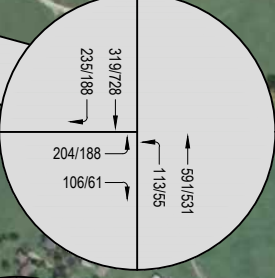
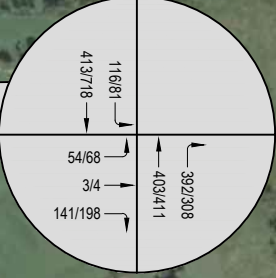
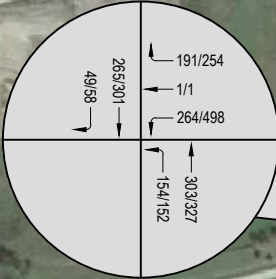
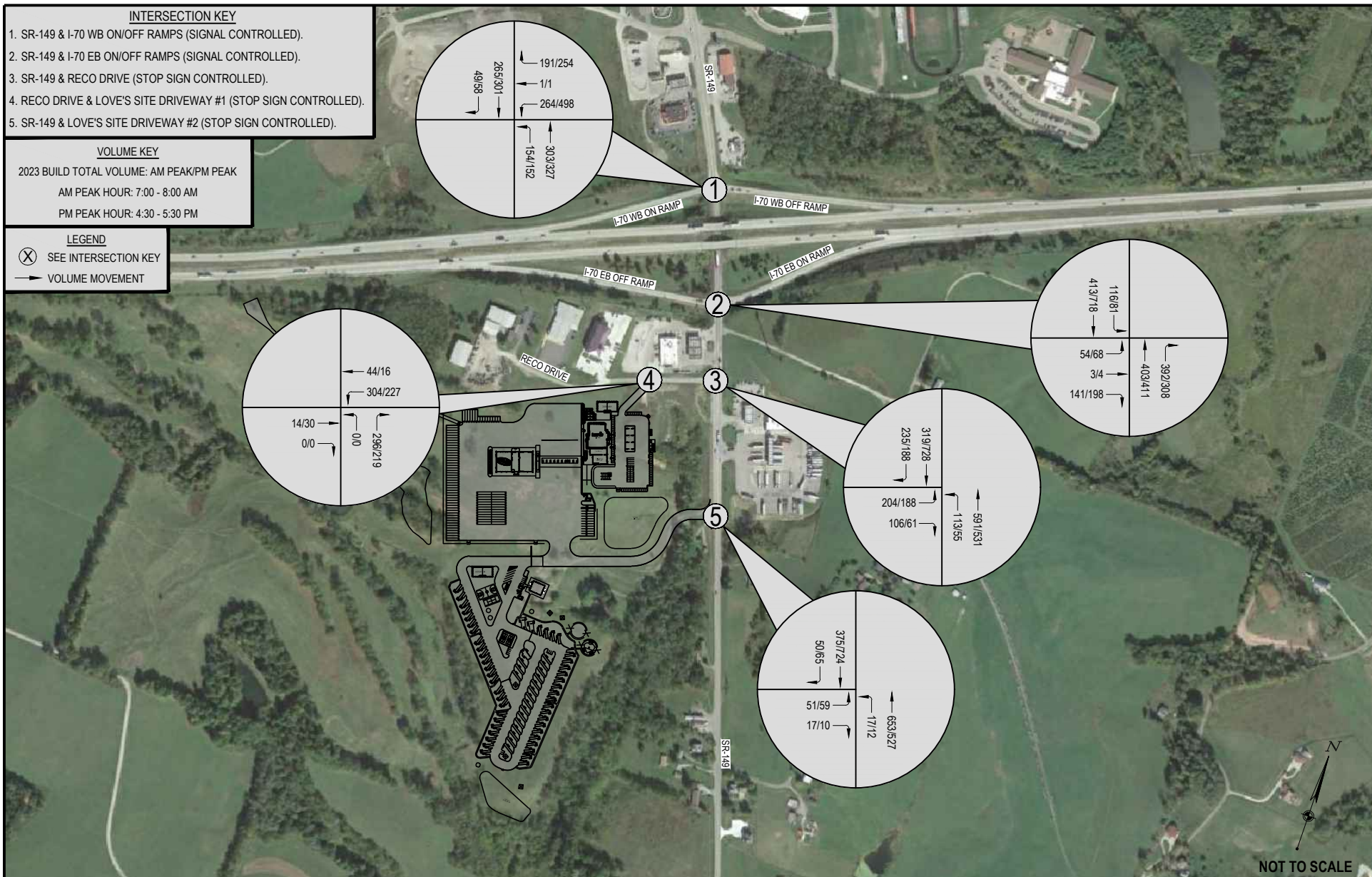
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

VOLUME KEY

2023 BUILD TOTAL VOLUME: AM PEAK/PM PEAK
 AM PEAK HOUR: 7:00 - 8:00 AM
 PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT



NOT TO SCALE

FIGURE 9.B
DATE: 11/21/2022
JOB NO.: 759267-02
DESIGNED BY: JMP
DRAWN BY: JMP
CHECKED BY: TMC
PAGE: 39

7. Estimates of 2043 No-Build Traffic in the Vicinity of the Site

7.1. 2043 No-Build Traffic Volumes

The 2043 No-Build Traffic Volumes – Cars and Trucks (Figure 10.A) were calculated by applying growth rates to the 2022 Design Hour Traffic Volumes (Figure 5.A). The growth rates were calculated using the linear regression method referenced in the *Ohio Traffic Forecasting Manual, Volume 2, Section 4.2* for all study roadways. A linear growth rate of 0.5 percent (%) was applied for twenty-one (21) years to the study roadways (growth factor of 1.105). The car and truck volumes shown on Figure 10.A were combined to form the 2043 No-Build Traffic Volumes – Total Volumes (Figure 10.B) for simplicity. The growth rate documentation is included in Appendix C of the report.

7.2. Capacity Analysis Parameters

The capacity of an intersection (signalized or unsignalized) can best be described by its corresponding Level of Service (LOS). The level of service of an intersection is a qualitative measure of the various attributes of an intersection. There are six levels of service ranging from “ideal” free flow conditions at LOS “A,” to forced or “breakdown” conditions at LOS “F.” The level of service for signalized intersections is based upon the average stopped delay per vehicle for various movements within the intersection. Although volume capacity ratio (v/c) affects delay, there are other parameters that more strongly affect it, such as the quality of progression, length of green phases, cycle lengths, and others. Thus, for any given v/c ratio, a range of delay values may result, and vice versa.

The level of service for unsignalized intersections is based upon total delay. Total delay is defined in the *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis*, as the total elapsed time from when a vehicle stops at the end of the queue until the vehicle departs from the stop line; this time includes the time required for the vehicle to travel from the last-in-queue position to the first-in-queue position. Table 3 summarizes the LOS definitions for unsignalized intersections. Throughout the report “unsignalized intersections” are commonly referred to as “stop sign controlled.”

Table 3
Level of Service Criteria (Unsignalized Intersections)

Level of Service	Delay per Vehicle (Sec.)	Description
A	≤ 10.0	Little or no delay.
B	> 10.0 and ≤ 15.0	Short traffic delays.
C	> 15.0 and ≤ 25.0	Average traffic delays.
D	> 25.0 and ≤ 35.0	Long traffic delays.
E	> 35.0 and ≤ 50.0	Very long traffic delays.
F	≥ 50.0	Extreme traffic delays.

Source: *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis*. Transportation Research Board.

Highway Capacity Manual 2016 (HCM 6th Edition) methodology was used in the Traffic Impact Study to remain consistent with “state-of-the-practice” professional standards. It is important to note that the Level of Service Criteria for unsignalized intersections is different than for signalized intersections. For example, a delay of 18 seconds yields level of service C under the unsignalized LOS criteria (see Table 3) while yielding level of service B under the signalized intersection LOS criteria (see Table 4). Table 4 summarizes the LOS definitions for signalized intersections.

**Table 4
 Level of Service Criteria (Signalized Intersections)**

Level of Service	Delay per Vehicle (Sec.)	Description
A	< 10.0	Most vehicles do not stop at all.
B	> 10.0 and ≤ 20.0	More vehicles stop than with LOS A.
C	> 20.0 and ≤ 35.0	The number of vehicles stopping is significant, although many pass through without stopping.
D	> 35.0 and ≤ 55.0	Many Vehicles stop. Individual cycle failures are noticeable.
E	> 55.0 and ≤ 80.0	Considered to be the limit of acceptable delay. Individual cycle failures are frequent.
F	> 80.0	Unacceptable delay.

Source: *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis*. Transportation Research Board.

Highway Capacity Software (HCS) Version 8.1 was utilized to calculate delay and level of service values. *ODOT Analysis and Traffic Simulation Manual (OATS)*, Section 6.2.2.1 was used for default saturation flow rate, arrival type, upstream filtering, initial queue, right turn on red value, and queue length percentile for all HCS intersection analyses. HCS Version 8.1 model parameters include traffic volumes, movements, heavy vehicle percentage, intersection traffic control, storage length, and lane widths. Per Section 5.2 of the OATS Manual, the peak hour factors (PHF) from the intersection TMCs were used and a PHF of 0.92 was used for the proposed site driveway intersections.

Per Section 8.1.1 of the OATS Manual, the capacity results presented in the tables below should be highlighted using the following color scheme:

- LOS D = Yellow
- LOS E = Orange
- LOS F = Red
- v/c ratio > 0.93 = Orange
- v/c ratio ≥ 1.0 = Red
- Queue-Storage Ratio (QSR) ≥ 1.0 = Red

Note: According to the ODOT Analysis and Traffic Simulation Manual (OATS), Section 5.9, the operational goals for intersection analyses (TWSC and Signalized) are to operate at overall LOS D or better conditions and for each movement to operate at LOS E or better conditions.

7.3. 2043 No-Build Traffic Scenario Capacity Analysis

Utilizing the 2043 No-Build Traffic Volumes illustrated on Figures 10.A-10.B, capacity calculations were performed for the key study intersections. Capacity calculations followed procedures documented in the *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis* (Transportation Research Board, 2016). All study intersections were analyzed with HCS Version 8.1 methodology.

Table 5 summarizes the capacity analyses results for the 2043 No-Build Traffic Scenario.

Table 5
Summary of 2043 No-Build Traffic Scenario Capacity Analysis

Lane	2043 AM Peak Hour No-Build					2043 PM Peak Hour No-Build				
	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)
SR-149 & I-70 WB On/Off Ramps										
Intersection	B	19.8	--	--	--	C	22.0	--	--	--
WBLT	C	27.1	0.570	0.16	183.2	D	35.1	0.866	0.30	343.5
WBR	D	36.5	0.765	0.71	248.5	C	22.2	0.653	0.57	199.5
WB Approach	C	32.2	--	--	--	C	29.9	--	--	--
NBLT	B	14.3	0.439	0.57	228.0	B	14.9	0.551	0.50	200.0
NB Approach	B	14.3	--	--	--	B	14.9	--	--	--
SBTR	B	10.2	0.408	0.24	152.4	B	14.4	0.507	0.29	184.2
SB Approach	B	10.2	--	--	--	B	14.4	--	--	--
SR-149 & I-70 EB On/Off Ramps										
Intersection	B	17.7	--	--	--	B	13.1	--	--	--
EBLT	C	20.3	0.162	0.03	42.8	C	22.9	0.310	0.04	57.0
EBR	C	20.8	0.234	0.25	62.5	C	27.4	0.602	0.51	127.5
EB Approach	C	20.6	--	--	--	C	25.7	--	--	--
NBT	B	11.6	0.376	0.31	145.7	A	8.0	0.363	0.22	103.4
NBR	B	14.3	0.528	0.49	200.9	A	7.7	0.321	0.19	77.9
NB Approach	B	13.1	--	--	--	A	7.8	--	--	--
SBLT	C	23.4	0.739	0.87	348.0	B	13.5	0.798	0.63	252.0
SB Approach	C	23.4	--	--	--	B	13.5	--	--	--
SR-149 & Reco Drive										
Intersection	--	--	--	--	--	--	--	--	--	--
EBLR	B	13.5	0.040	--	2.5	D	27.0	0.180	--	15.0
NBLT	A	8.3	0.010	--	0.0	A	9.8	0.010	--	0.0

L – Left T – Through R – Right

Under the **2043 No-Build Traffic Scenario**, the signal-controlled intersections operate at overall level of service (LOS) “C” or better conditions. In addition, the individual movements of the stop-controlled intersection operate at level of service (LOS) “D” or better conditions.

The 2043 No-Build Traffic Scenario Capacity Analysis Summary Sheets are contained in Appendix E of the report.

INTERSECTION KEY

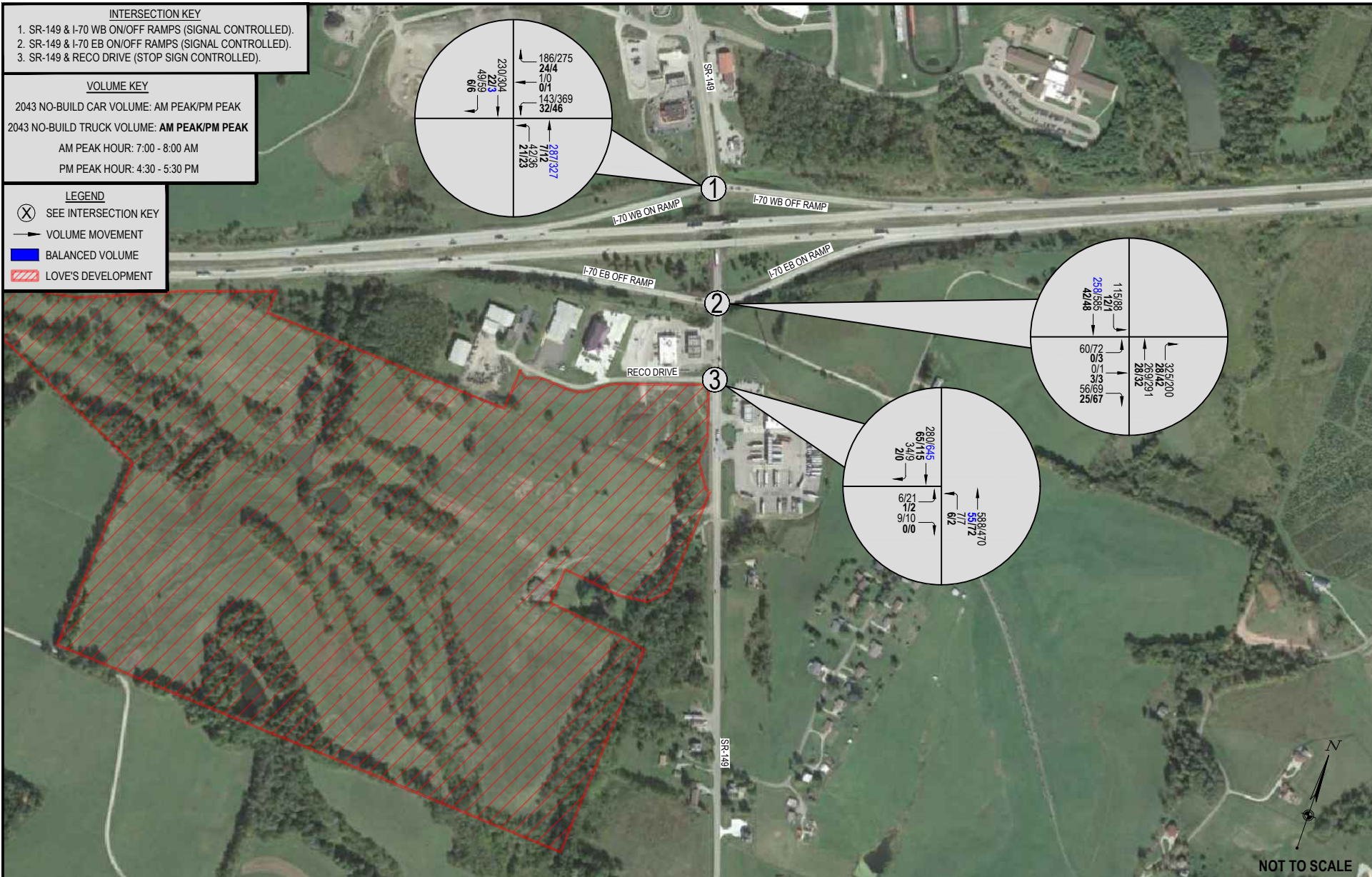
1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

2043 NO-BUILD CAR VOLUME: AM PEAK/PM PEAK
 2043 NO-BUILD TRUCK VOLUME: **AM PEAK/PM PEAK**
 AM PEAK HOUR: 7:00 - 8:00 AM
 PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

INTERSECTION KEY

1. SR-149 & I-70 WB ON/OFF RAMP (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMP (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).

VOLUME KEY

2043 NO-BUILD TOTAL VOLUME: AM PEAK/PM PEAK

AM PEAK HOUR: 7:00 - 8:00 AM

PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT
- BALANCED VOLUME
- ▨ LOVE'S DEVELOPMENT



NOT TO SCALE

8. Estimates of 2043 Design Year Traffic in the Vicinity of the Site

8.1. 2043 Design Year Traffic Volumes

The 2043 Design Year Traffic Volumes (Figures 11.A-11.B) were calculated by adding the Site Generated Traffic Volumes (Figures 8.A-8.C) to the 2043 No-Build Traffic Volumes – Cars and Trucks (Figure 10.A). The car and truck volumes shown on Figure 11.A were combined to form the 2043 Design Year Traffic Volumes – Total Volumes (Figure 11.B) for simplicity.

8.2. 2043 Design Year Traffic Scenario Capacity Analysis – Build Condition 1 and Build Condition 2

Utilizing the 2043 Design Year Traffic Volumes illustrated on Figures 11.A-11.B, capacity calculations were performed for the Site driveways and key study intersections. Capacity calculations followed procedures documented in the *Highway Capacity Manual, Sixth Edition: A Guide for Multimodal Mobility Analysis* (Transportation Research Board, 2016). All study intersections were analyzed with HCS Version 8.1 methodology. Tables 6 and 7 summarize the capacity analyses results for the 2043 Design Year Traffic Scenario Condition 1 (existing geometry) and Condition 2 (with improvements). *NOTE: Table 7 only shows the capacity analysis results for the intersections that were impacted by the recommended improvements.*

Table 6
Summary of 2043 Design Year Traffic Scenario Capacity Analysis – Build Condition 1

Lane	2043 AM Peak Hour Design Year					2043 PM Peak Hour Design Year				
	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)
SR-149 & I-70 WB On/Off Ramps										
Intersection	C	33.0	--	--	--	E	61.3	--	--	--
WBLT	E	57.3	0.927	0.35	400.8	F	109.0	1.145	0.70	801.5
WBR	D	37.8	0.779	0.74	259.0	C	22.8	0.670	0.59	206.5
WB Approach	D	49.0	--	--	--	E	79.5	--	--	--
NBLT	C	32.4	0.803	1.11	444.0	F	68.2	1.035	1.45	580.0
NB Approach	C	32.4	--	--	--	E	68.2	--	--	--
SBTR	B	10.9	0.462	0.28	177.8	B	15.0	0.540	0.32	203.2
SB Approach	B	10.9	--	--	--	B	15.0	--	--	--
SR-149 & I-70 EB On/Off Ramps										
Intersection	E	59.7	--	--	--	C	22.5	--	--	--
EBLT	C	20.3	0.162	0.03	42.8	C	23.0	0.321	0.05	71.3
EBR	C	22.4	0.428	0.49	122.5	E	75.4	0.965	1.41	352.5
EB Approach	C	21.7	--	--	--	E	61.1	--	--	--
NBT	B	14.6	0.564	0.53	249.1	A	9.8	0.513	0.36	169.2
NBR	B	17.5	0.657	0.66	270.6	A	9.3	0.453	0.30	123.0
NB Approach	B	16.1	--	--	--	A	9.6	--	--	--
SBLT	F	132.3	1.205	2.81	1124.0	C	21.0	0.939	0.97	388.0
SB Approach	F	132.3	--	--	--	C	21.0	--	--	--
SR-149 & Reco Drive										
Intersection	--	--	--	--	--	--	--	--	--	--
EBLR	F	356.3	1.660	--	572.5	F	630.5	2.220	--	575.0
NBLT	A	9.8	0.150	--	12.5	B	11.8	0.100	--	7.5

L – Left T – Through R – Right

Table 6 – Continued.
Summary of 2043 Design Year Traffic Scenario Capacity Analysis – Build Condition 1

Lane	2043 AM Peak Hour Design Year					2043 PM Peak Hour Design Year				
	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)
Reco Drive & Love’s Site Driveway #1										
Intersection	--	--	--	--	--	--	--	--	--	--
WBLT	A	7.8	0.210	--	20.0	A	7.7	0.160	--	15.0
NBLR	A	9.8	0.300	--	32.5	A	9.5	0.230	--	22.5
SR-149 & Love’s Site Driveway #2										
Intersection	--	--	--	--	--	--	--	--	--	--
EBLR	F	58.0	0.540	--	65.0	F	170.6	0.930	--	125.0
NBLT	A	8.6	0.020	--	2.5	B	10.4	0.020	--	2.5
L – Left T – Through R – Right										

Under the **2043 Design Year Traffic Scenario Build Condition 1**, the signal-controlled intersections operate at overall level of service (LOS) “E” or better conditions. In addition, the individual movements of the stop-controlled intersections operate at level of service (LOS) “B” or better conditions with the exception of the following:

- The EBLR movement at the SR-149 & Reco Drive intersection which operates at LOS “F” during both the AM and PM Peak Hours.
- The EBLR movement at the SR-149 & Love’s Site Driveway #2 intersection which operates at LOS “F” during both the AM and PM Peak Hours.

Table 7
Summary of 2043 Design Year Traffic Scenario Capacity Analysis – Build Condition 2

Lane	2043 AM Peak Hour Design Year					2043 PM Peak Hour Design Year				
	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)
SR-149 & I-70 WB On/Off Ramps										
Intersection	C	20.1	--	--	--	C	32.7	--	--	--
WBLT	C	30.7	0.885	0.26	297.7	D	42.2	0.947	0.42	480.9
WBR	C	27.8	0.743	0.63	220.5	B	16.4	0.554	0.48	168.0
WB Approach	C	29.4	--	--	--	C	33.3	--	--	--
NBLT	B	14.0	0.512	0.47	188.0	D	45.2	0.886	0.89	356.0
NB Approach	B	14.0	--	--	--	D	45.2	--	--	--
SBTR	B	11.6	0.471	0.30	190.5	C	20.4	0.631	0.37	235.0
SB Approach	B	11.6	--	--	--	C	20.4	--	--	--
SR-149 & I-70 EB On/Off Ramps										
Intersection	B	18.0	--	--	--	C	28.6	--	--	--
EBLT	C	30.1	0.315	0.04	57.0	C	21.8	0.293	0.04	57.0
EBR	D	36.1	0.834	0.66	165.0	C	29.4	0.883	0.85	212.5
EB Approach	C	34.4	--	--	--	C	27.4	--	--	--
NBT	A	8.7	0.250	0.44	110.0	B	14.9	0.343	0.75	187.5
NBR	A	5.5	0.292	0.26	65.0	B	12.1	0.303	0.45	112.5
NB Approach	A	7.1	--	--	--	B	13.7	--	--	--
SBLT	C	20.9	0.711	1.17	468.0	F	37.3	1.032	0.95	380.0
SB Approach	C	20.9	--	--	--	D	37.3	--	--	--
L – Left T – Through R – Right										

Table 7 – Continued.
Summary of 2043 Design Year Traffic Scenario Capacity Analysis – Build Condition 2

Lane	2043 AM Peak Hour Design Year					2043 PM Peak Hour Design Year				
	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)	LOS	Delay (sec/veh)	v/c	QSR	95 th %ile Queue (ft)
SR-149 & Reco Drive										
Intersection	C	21.3	--	--	--	D	54.6	--	--	--
EBLTR	D	48.8	0.907	0.65	325.0	D	49.1	0.878	0.51	255.0
EB Approach	D	48.8	--	--	--	D	49.1	--	--	--
NBL	B	12.3	0.330	0.26	45.5	B	15.9	0.302	0.12	21.0
NBT	A	8.8	0.337	0.67	117.3	A	7.3	0.359	0.55	96.3
NBR	B	11.4	0.513	1.03	180.3	A	7.6	0.375	0.51	89.3
NB Approach	B	10.6	--	--	--	A	8.2	--	--	--
SBT	C	24.3	0.647	1.25	312.5	F	103.3	1.179	3.87	967.5
SBR	B	15.6	0.517	0.78	156.0	B	17.1	0.330	0.51	102.0
SB Approach	C	20.8	--	--	--	F	86.8	--	--	--
SR-149 & Love’s Site Driveway #2										
Intersection	--	--	--	--	--	--	--	--	--	--
EBLR	F	52.8	0.510	--	60.0	F	147.7	0.870	--	115.0
NBL	A	8.6	0.020	--	2.5	B	10.4	0.020	--	2.5

L – Left T – Through R – Right

Under the **2043 Design Year Traffic Scenario Build Condition 2**, the signal-controlled intersections operate at overall level of service (LOS) “D” or better conditions. In addition, the individual movements of the stop-controlled intersection operate at level of service (LOS) “B” or better conditions with the exception of the following:

- The EBLR movement at the SR-149 & Love’s Site Driveway #2 intersection which operates at LOS “F” during both the AM and PM Peak Hours.

The recommended improvements include the following:

- The installation of a traffic signal at the intersection of SR-149 & Reco Drive, coordinated with the signal-controlled intersections of SR-149 & I-70 WB On/Off Ramps, SR-149 & I-70 EB On/Off Ramps, and SR-149 & Bond Drive.
- Restripe the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane back to Pilot’s South Driveway.
- Construction of a NB to WB left-turn lane at the intersection of SR-149 & Love’s Site Driveway #2.
- Construction of a SB to WB right-turn lane at both intersections of SR-149 & Reco Drive and SR-149 & Love’s Site Driveway #2.
- Signal timing splits and offsets were optimized at the signal-controlled intersections.

The 2043 Design Year Traffic Scenario Capacity Analysis Summary Sheets (Build Condition 1 and Build Condition 2) are contained in Appendix F of the report.

INTERSECTION KEY

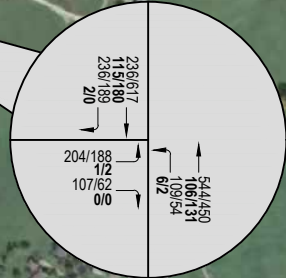
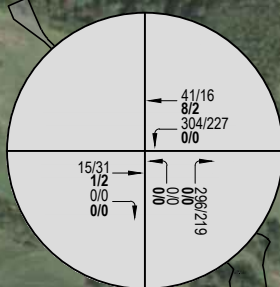
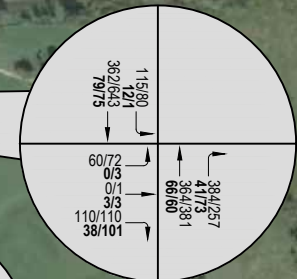
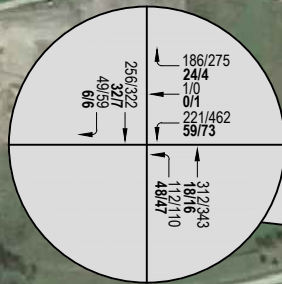
1. SR-149 & I-70 WB ON/OFF RAMPs (SIGNAL CONTROLLED).
2. SR-149 & I-70 EB ON/OFF RAMPs (SIGNAL CONTROLLED).
3. SR-149 & RECO DRIVE (STOP SIGN CONTROLLED).
4. RECO DRIVE & LOVE'S SITE DRIVEWAY #1 (STOP SIGN CONTROLLED).
5. SR-149 & LOVE'S SITE DRIVEWAY #2 (STOP SIGN CONTROLLED).

VOLUME KEY

2043 BUILD CAR VOLUME: AM PEAK/PM PEAK
 2043 BUILD TRUCK VOLUME: **AM PEAK/PM PEAK**
 AM PEAK HOUR: 7:00 - 8:00 AM
 PM PEAK HOUR: 4:30 - 5:30 PM

LEGEND

- ⊗ SEE INTERSECTION KEY
- VOLUME MOVEMENT



NOT TO SCALE



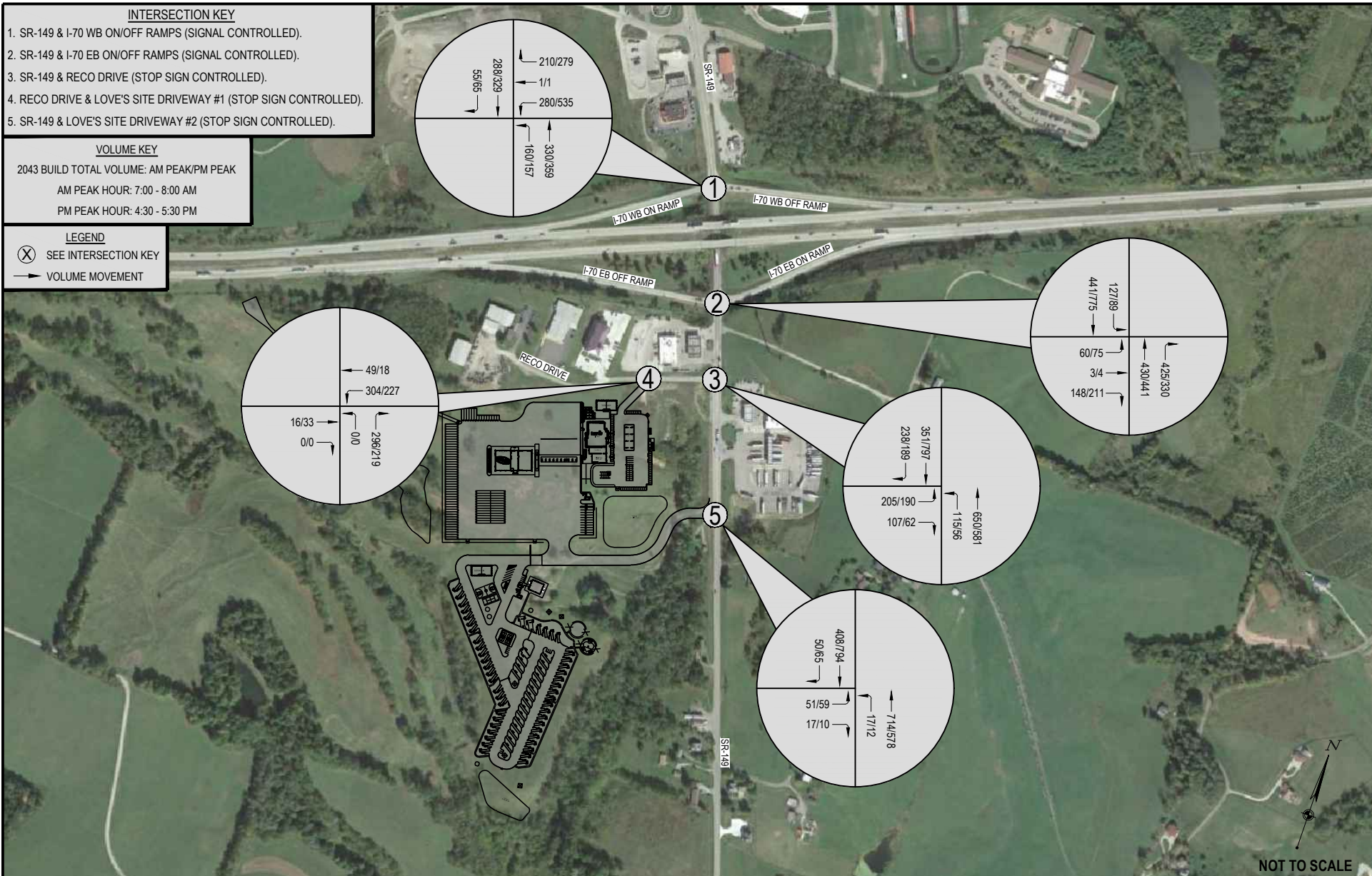
2043 DESIGN YEAR TRAFFIC VOLUMES - CARS & TRUCKS

LOVE'S DEVELOPMENT

UNION TOWNSHIP

BELMONT, OHIO

FIGURE 11.A
DATE: 11/21/2022
JOB NO.: 759267-02
DESIGNED BY: JMP
DRAWN BY: JMP
CHECKED BY: TMC
PAGE: 48



NOT TO SCALE

9. Turn Lane Analysis

Left and right-turn lane analyses were completed using the turn lane warrant charts from the ODOT *Location & Design Manual – Volume I (July 2022)* and capacity analysis results. Based on a speed limit of 45 mph (50 mph Design Speed) on SR-149, the high-speed turn lane warrant charts were used for the analysis.

9.1. Left-Turn Lane Analysis

Table 8 provides a summary of the data and results utilized in the review of each study location for a left-turn lane. ODOT Chart 401-5b and capacity analysis results were used to determine if left-turn lanes were warranted at the applicable study locations.

Table 8
Left-Turn Lane Warrant Review

Intersection and Traffic Scenario	Lane	*Advancing Traffic (am/pm)	Opposing Traffic (am/pm)	Left-Turn (am/pm)	% Left	Method or Chart Used	Warranted
2023 No-Build Traffic Scenario							
SR-149 & I-70 WB On/Off Ramps	NBL	324/361	278/337	57/54	17.6%/15.0%	Capacity	No/No
SR-149 & I-70 EB On/Off Ramps	SBL	388/657	590/513	116/81	29.9%/12.3%	Capacity	No/No
2023 Build Traffic Scenario							
SR-149 & I-70 WB On/Off Ramps	NBL	457/479	314/359	154/152	33.7%/31.7%	Capacity	No/Yes
SR-149 & I-70 EB On/Off Ramps	SBL	529/799	795/719	116/81	21.9%/10.1%	Capacity	Yes/No
SR-149 & Reco Drive	NBL	704/586	554/916	113/55	16.1%/9.4%	401-5b	Yes/Yes
SR-149 & Love’s Site Driveway #2	NBL	670/539	425/789	17/12	2.5%/2.2%	401-5b	Yes/Yes
2043 No-Build Traffic Scenario							
SR-149 & I-70 WB On/Off Ramps	NBL	357/398	307/372	63/59	17.7%/14.8%	Capacity	No/No
SR-149 & I-70 EB On/Off Ramps	SBL	427/722	650/565	127/89	29.7%/12.3%	Capacity	No/No
2043 Design Year Traffic Scenario							
SR-149 & I-70 WB On/Off Ramps	NBL	490/516	343/394	160/157	32.7%/30.4%	Capacity	Yes/Yes
SR-149 & I-70 EB On/Off Ramps	SBL	568/864	855/771	127/89	22.4%/10.3%	Capacity	Yes/No
SR-149 & Reco Drive	NBL	765/637	589/986	115/56	15.0%/8.8%	401-5b	Yes/Yes
SR-149 & Love’s Site Driveway #2	NBL	731/590	458/859	17/12	2.3%/2.0%	401-5b	Yes/Yes

* Includes Left Turns

9.2. Left-Turn Lane Warrant Review Summary

According to ODOT Chart 401-5b, a NB to WB left-turn lane **is warranted** at the intersections of SR-149 & Reco Drive, and SR-149 & Love’s Site Driveway #2 under the 2023 Build Traffic Scenario. According to capacity analysis results, a NB to WB left-turn lane **is warranted** at the intersection of SR-149 & I-70 WB On/Off Ramps during the 2023 Build Traffic Scenario and a SB to EB left-turn lane **is warranted** at the intersection of SR-149 & I-70 EB On/Off Ramps during the 2023 Build Traffic Scenario. However, due to the existing overpass and location of the existing bridge piers, there are physical limitations to being able to widen SR-149 and provide the additional turn lanes needed.

9.3. Right-Turn Lane Analysis

Table 9 provides a summary of the data and results utilized in the review of each study location for a right-turn lane. ODOT Chart 401-6b was used to determine if right-turn lanes were warranted at the applicable study locations.

**Table 9
 Right-Turn Lane Warrant Review**

Intersection and Traffic Scenario	Direction	*Advancing Traffic (am/pm)	Right-Turn (am/pm)	Method or Chart Used	Warranted
2023 Build Traffic Scenario					
SR-149 & Reco Drive	SBR	554/916	235/188	401-6b	Yes/Yes
SR-149 & Love’s Site Driveway #2	SBR	425/789	50/65	401-6b	Yes/Yes
2043 Design Year Traffic Scenario					
SR-149 & Reco Drive	SBR	589/986	238/189	401-6b	Yes/Yes
SR-149 & Love’s Site Driveway #2	SBR	458/859	50/65	401-6b	Yes/Yes

* Includes Right Turns

9.4. Right-Turn Lane Warrant Review Summary

According to ODOT Chart 401-6b a SB to WB right-turn lane **is warranted** at the intersections of SR-149 & Reco Drive and SR-149 & Love’s Site Driveway #2 under the 2023 Build Traffic Scenario.

ODOT Turn Lane Resources are located in Appendix G of the report.

9.5. Turn Lane Storage Lengths

Turn lane storage length calculations were completed based upon procedures in the ODOT *Location & Design Manual – Volume I, Section 400 (July 2022)*. Specifically, ODOT sheet 401-9 – Basis for Computing Length of Turn Lanes and sheet 401-10 – Storage Length at Intersections were used. The turn lane length analysis was conducted to provide the final design length of existing/proposed turn lanes under the 2043 Design Year Traffic Scenario. Table 10 summarizes the storage lengths required for the AM Peak Hour while Table 11 summarizes the storage lengths required for the PM Peak Hour. ODOT Storage Length Calculation Sheets are located in Appendix G of the report.

Table 10
Storage Length Analysis - AM Peak Hour

Intersection	Direction	DHV	No. of Lanes	Cycles/ Hour	Avg. Veh/ Cycle/ Lane	Design Speed (mph)	Fig. 401-10 Storage Length (ft)	Fig. 401-9 Condition			Backup Length (ft)	Required Turn Lane Length* (ft)	Proposed Storage Length* (ft)	Turn Lane Length Ex/Prop < Required	HCS 95 th % Queue Length** (ft)
								A*	B*	C*					
2043 Design Year Traffic Scenario															
SR-149 & I-70 WB On/Off Ramps	NBL	160	1	45	4.0	50	175	--	225	320	--	320	375	Yes	188.0
	NBT	330	1	45	8.0	50	325	--	--	--	325	--	--	--	188.0
SR-149 & I-70 EB On/Off Ramps	SBL	127	1	45	3.0	50	150	--	225	295	--	295	425	Yes	468.0
	SBT	441	1	45	10.0	50	375	--	--	--	375	--	--	--	468.0
SR-149 & Reco Drive	NBL	115	1	45	3.0	50	150	--	225	295	--	295	575	Yes	45.5
	NBT	650	1	45	15.0	50	525	--	--	--	525	--	--	--	117.3
	SBT	351	1	45	8.0	50	325	--	--	--	325	--	--	--	312.5
	SBR	238	1	45	6.0	50	250	--	225	395	--	395	395	Yes	156.0
SR-149 & Love’s Site Driveway #2	NBL	17	1	60	1.0	50	50	--	225	--	--	225	225	No	2.5
	SBR	50	1	60	1.0	50	50	--	225	195	--	225	225	No	0.0

* Includes 50' Diverging Taper.

Table 11
Storage Length Analysis - PM Peak Hour

Intersection	Direction	DHV	No. of Lanes	Cycles/ Hour	Avg. Veh/ Cycle/ Lane	Design Speed (mph)	Fig. 401-10 Storage Length (ft)	Fig. 401-9 Condition			Backup Length (ft)	Required Turn Lane Length* (ft)	Proposed Storage Length* (ft)	Turn Lane Length Ex/Prop < Required	HCS 95 th % Queue Length** (ft)
								A*	B*	C*					
2043 Design Year Traffic Scenario															
SR-149 & I-70 WB On/Off Ramps	NBL	157	1	51	4.0	50	175	--	225	320	--	320	375	Yes	356.0
	NBT	359	1	51	8.0	50	325	--	--	--	325	--	--	--	356.0
SR-149 & I-70 EB On/Off Ramps	SBL	89	1	51	2.0	50	100	--	225	245	--	245	600	Yes	380.0
	SBT	775	1	51	16.0	50	550	--	--	--	550	--	--	--	380.0
SR-149 & Reco Drive	NBL	56	1	51	2.0	50	100	--	225	245	--	245	500	Yes	21.0
	NBT	581	1	51	12.0	50	450	--	--	--	450	--	--	--	96.3
	SBT	797	1	51	16.0	50	550	--	--	--	550	--	--	--	967.5
	SBR	189	1	51	4.0	50	175	--	225	320	--	320	600	Yes	102.0
SR-149 & Love’s Site Driveway #2	NBL	12	1	60	1.0	50	50	--	225	--	--	225	225	No	2.5
	SBR	65	1	60	2.0	50	100	--	225	--	--	225	225	No	0.0

* Includes 50' Diverging Taper.

10. Queue Length Analysis

10.1. Queue Length Analysis Procedure and Results

The 95th percentile queue lengths were calculated using Highway Capacity Software (HCS) Version 8.1. CESO reviewed the 2043 No-Build and 2043 Design Year Build Traffic Scenarios. The results of the analyses are listed below in Tables 12.A-12.B. The 95th percentile queue length analysis summary sheets are located in Appendix H of the report.

Table 12.A
Queue Length Analysis – 2043 No-Build and Design Year Traffic Scenarios – AM Peak Hour

Location	Movement	Existing [Proposed] Storage Length	2043 No-Build/Design Year Traffic Scenario Comparison		
			95 th Percentile Queue Lengths		
			AM Peak Hour		
Traffic Scenario →			2043 NB	2043 BD Cond 1	2043 BD Cond 2
SR-149 & I-70 WB On/Off Ramps ²	WBLT	---	183.2	400.8	297.7
	WBR	350'	248.5	259.0	220.5
	NBLT	---	228.0	444.0	188.0
	SBTR	---	152.4	177.8	190.5
SR-149 & I-70 EB On/Off Ramps ²	EBLT	---	42.8	42.8	57.0
	EBR	250'	62.5	122.5	165.0
	NBT	---	145.7	249.1	110.0
	NBR	410'	200.9	270.6	65.0
SR-149 & Reco Drive ¹	SBLT	---	348.0	1124.0	468.0
	EBLR	---	2.5	572.5	
	EBLTR	---			325.0
	NBLT	---	0.0	12.5	
	[NBL]	[TWLTL]			45.5
	[NBT]	---			117.3
	NBR	---			180.3
	[SBT]	---			312.5
[SBR]	[200']			156.0	
Reco Drive & Love’s Site Driveway #1 ¹	WBLT	---		20.0	
	NBLR	---		32.5	
SR-149 & Love’s Site Driveway #2 ¹	EBLR	---		65.0	60.0
	NBLT	---		2.5	
	[NBL]	[175']			2.5

¹ – Value calculated by multiplying value by an average car length of 25 feet to provide a more accurate result.

² – Value calculated by multiplying the Queue-Storage Ratio by the storage length.

Table 12.B
Queue Length Analysis – 2043 No-Build and Design Year Traffic Scenarios – PM Peak Hour

Location	Movement	Existing [Proposed] Storage Length	2043 No-Build/Design Year Traffic Scenario Comparison		
			95 th Percentile Queue Lengths		
			PM Peak Hour		
Traffic Scenario →			2043 NB	2043 BD Cond 1	2043 BD Cond 2
SR-149 & I-70 WB On/Off Ramps ²	WBLT	---	343.5	801.5	480.9
	WBR	350'	199.5	206.5	168.0
	NBLT	400'	200.0	580.0	356.0
	SBTR	---	184.2	203.2	235.0
SR-149 & I-70 EB On/Off Ramps ²	EBLT	---	57.0	71.3	57.0
	EBR	250'	127.5	352.5	212.5
	NBT	---	103.4	169.2	187.5
	NBR	410'	77.9	123.0	112.5
	SBLT	400'	252.0	388.0	380.0
SR-149 & Reco Drive ¹	EBLR	---	15.0	575.0	
	EBLTR	---			255.0
	NBLT	---	0.0	7.5	
	[NBL]	[TWLTL]			21.0
	[NBT]	---			96.3
	NBR	---			89.3
	[SBT]	---			967.5
	[SBR]	[200']			102.0
Reco Drive & Love’s Site Driveway #1 ¹	WBLT	---		15.0	
	NBLR	---		22.5	
SR-149 & Love’s Site Driveway #2 ¹	EBLR	---		125.0	115.0
	NBLT	---		2.5	
	[NBL]	[175']			2.5

¹ – Value calculated by multiplying value by an average car length of 25 feet to provide a more accurate result.

² – Value calculated by multiplying the Queue-Storage Ratio by the storage length.

10.2. Queue Length Analysis Summary

Based on Tables 12.A-12.B, the following conclusions were found:

- The ODOT required storage length for the NBL turn lane at the SR-149 & Reco Drive intersection is 525’ of storage plus a 50’ taper. However, CESO recommends restriping the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane (TWLTL).
- The ODOT required storage length for the SBR turn lane at the SR-149 & Reco Drive intersection is 550’ of storage plus a 50’ taper. However, due to close proximity to the existing I-70 EB Off Ramp, CESO recommends constructing the turn lane to accommodate 200’ of storage plus a 50’ taper.
- The ODOT required storage length for the NBL turn lane at the SR-149 & Love’s Site Driveway #2 intersection is 175’ of storage plus a 50’ taper. CESO recommends constructing the turn lane to accommodate 175’ of storage plus a 50’ taper.
- The ODOT required storage length for the SBR turn lane at the SR-149 & Love’s Site Driveway #2 intersection is 175’ of storage plus a 50’ taper. CESO recommends constructing the turn lane to accommodate 175’ of storage plus a 50’ taper.

11. Traffic Signal Warrant Study

The following Traffic Signal Warrant Study was performed according to the specifications stated in the *Ohio Manual of Uniform Traffic Control Devices (OMUTCD)*, Section 4C, for the intersection of SR-149 & Reco Drive.

11.1. Traffic Signal Warrants

In accordance with the OMUTCD, Chapter 4C, the study intersections were tested against the following nine (9) warrants:

- Warrant 1 – Eight-Hour Vehicular Volume.
- Warrant 2 – Four-Hour Vehicular Volume.
- Warrant 3 – Peak Hour.
- Warrant 4 – Pedestrian Volume.
- Warrant 5 – School Crossing.
- Warrant 6 – Coordinated Signal System.
- Warrant 7 – Crash Experience.
- Warrant 8 – Roadway Network.
- Warrant 9 – Intersection near a Grade Crossing.

While meeting one or more warrants is not considered sufficient justification for the installation of a traffic signal, it is necessary to do so before a signal can be considered.

Examination of the 9 warrants shows that not all warrants are appropriate for consideration at the studied locations. For example, it would not be appropriate to consider Warrant 4 – Pedestrian Volume or Warrant 9 – Intersection near a Grade Crossing at this location. **The specific warrants that were analyzed include Warrants 1, 2, and 3.** These warrants are discussed in the sections below.

The volumes used in the Traffic Signal Warrant Study were generated utilizing data contained in the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*, in combination with methods outlined in the (ITE) *Trip Generation Handbook*. The generated trips exiting at Reco Drive are distributed based on the percentages on Figures 7.A-7.C. The right-turn volumes exiting Reco Drive were reduced based on ODOT *Traffic Engineering Manual* (Section 402-5) methodologies. The signal warrant volumes are listed below in Table 13.

Table 13
Signal Warrant Volumes (Major and Minor Street)

Time	2023 No-Build		2023 Build		2043 No-Build		2043 Design Year	
	Major	Minor	Major	Minor	Major	Minor	Major	Minor
Intersection →	SR-149 & Reco Drive							
6:00 am – 7:00 am	542	15	676	106	596	17	730	107
7:00 am – 8:00 am	869	9	1023	137	955	10	1109	138
8:00 am – 9:00 am	746	20	929	161	820	22	1003	163
11:00 am – 12:00 pm	579	23	837	199	637	25	895	202
12:00 pm – 1:00 pm	705	20	1015	264	775	22	1085	266
1:00 pm – 2:00 pm	670	16	931	209	737	18	997	211
2:00 pm – 3:00 pm	782	17	1000	195	860	19	1078	197
3:00 pm – 4:00 pm	890	30	1136	220	979	33	1224	223
4:00 pm – 5:00 pm	894	36	1118	205	983	41	1207	211
5:00 pm – 6:00 pm	924	23	1161	207	1016	25	1253	209

It should be noted that according to the ODOT TEM Section 402-3.2, for new ODOT signals, Warrants 1, 2, and 3 shall be based on the 100 percent (%) values (OMUTCD Chapter 4C) and right-turn reduction factors applied.

11.2. Warrant 1 – Eight Hour Vehicular Volume

According to the OMUTCD, two conditions need to be examined to determine if this warrant is satisfied. The Minimum Vehicular Volume condition (Condition A) "... is intended for application where a large volume of intersecting traffic is the principal reason to consider installing a traffic control signal." The Interruption of Continuous Traffic condition (Condition B) "... is intended for application where the traffic volume on a major street is so heavy that traffic on a minor intersecting street suffers excessive delay or conflict in entering or crossing the major street."

It is intended that Warrant 1 be treated as a single warrant. If Condition A is satisfied, then Warrant 1 is satisfied and analyses of Condition B and the combination of Conditions A and B are not needed. Similarly, if Condition B is satisfied, then Warrant 1 is satisfied and an analysis of the combination of Conditions A and B is not needed.

The following conditions must exist for any eight (8) hours of an average day for a traffic control signal to be considered:

2023 and 2043 No-Build – SR-149 and Reco Drive (1 Lane Major/1 Lane Minor):

- Condition A: The major street traffic volume must exceed 500 vehicles (total of both approaches) per hour, while the minor street traffic volume must exceed 150 vehicles (higher volume approach) for the same eight (8) hours.
- Condition B: The major street traffic volume must exceed 750 vehicles (total of both approaches) per hour, while the minor street traffic volume must exceed 75 vehicles (higher volume approach) for the same eight (8) hours.
- Conditions A and B can be combined for locations where neither Condition A nor Condition B is satisfied. However, this combination should only be applied after a trial of other alternatives has failed to solve the traffic problems.

2023 Build and 2043 Design Year – SR-149 and Reco Drive (2 Lanes Major/1 Lane Minor):

- Condition A: The major street traffic volume must exceed 600 vehicles (total of both approaches) per hour, while the minor street traffic volume must exceed 150 vehicles (higher volume approach) for the same eight (8) hours.
- Condition B: The major street traffic volume must exceed 900 vehicles (total of both approaches) per hour, while the minor street traffic volume must exceed 75 vehicles (higher volume approach) for the same eight (8) hours.
- Conditions A and B can be combined for locations where neither Condition A nor Condition B is satisfied. However, this combination should only be applied after a trial of other alternatives has failed to solve the traffic problems.

Table 14 lists the Minimum Vehicular Volumes for Warrant 1 (Table 4C-1 from the OMUTCD).

Table 14
Minimum Vehicular Volumes for Warrant 1 (Table 4C-1 from the OMUTCD)

Condition A—Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B—Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on higher-volume minor-street approach (one direction only)			
Major Street	Minor Street	100% ^a	80% ^b	70% ^c	56% ^d	100% ^a	80% ^b	70% ^c	56% ^d
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

- ^a Basic minimum hourly volume
- ^b Used for combination of Conditions A and B after adequate trial of other remedial measures
- ^c May be used when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000
- ^d May be used for combination of Conditions A and B after adequate trial of other remedial measures when the major-street speed exceeds 40 mph or in an isolated community with a population of less than 10,000

The traffic volumes in Table 13 were tested against the volume thresholds listed in Table 14 Minimum Vehicular Volumes for Warrant 1 (Table 4C-1 from the OMUTCD). Table 15 presents the results for the analysis of Warrant 1 – Eight-Hour Vehicular Volume.

Table 15
Summary of Warrant 1 – Eight-Hour Vehicular Volume

Traffic Scenario	Study Year	Condition A			Condition B		
		Hours Satisfied		Condition Satisfied	Hours Satisfied		Condition Satisfied
		Major Street	Minor Street		Major Street	Minor Street	
SR-149 & Reco Drive							
2023 No-Build Traffic Scenario	2023	10	0	No	5	0	No
2023 Build Traffic Scenario	2023	10	8	Yes	8	10	Yes
2043 No-Build Traffic Scenario	2043	10	0	No	7	0	No
2043 Design Year Traffic Scenario	2043	10	8	Yes	8	10	Yes

11.3. Warrant 2 – Four Hour Vehicular Volume

According to the OMUTCD, this warrant is “...intended to be applied where the volume of intersecting traffic is the principal reason to consider installing a traffic control signal.” Warrant 2 is satisfied when the vehicular volumes (summarized in Table 13) in each of four (4) hours of an average day fall above the appropriate curve of the graphs labeled as Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume. Table 16 presents the results for the analysis of Warrant 2 – Four-Hour Vehicular Volume.

Table 16
Summary of Warrant 2 – Four-Hour Vehicular Volume

Traffic Scenario	Study Year	No. of Plotted Points That Fall Above the Appropriate Line	Warrant Satisfied
SR-149 & Reco Drive			
2023 No-Build	2023	0	No
2023 Build	2023	7	Yes
2043 No-Build	2043	0	No
2043 Design Year	2043	9	Yes

11.4. Warrant 3 – Peak Hour Volume

According to the OMUTCD, this warrant is “...intended for use at a location where traffic conditions are such that for a minimum of 1 hour of an average day, the minor-street traffic suffers undue delay when entering or crossing the major street.”

A traffic control signal shall be considered if the criteria in either of the following two conditions are met:

- A. If all three of the following conditions exist for the same one (1) hour (any four consecutive 15-minute periods) of an average day:
 - The total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a stop sign equals or exceeds 4 vehicle-hours for a one-lane approach or 5 vehicle-hours for a two-lane approach, and;
 - The volume on the same minor-street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes, and;
 - The total entering volume serviced during the hour equals or exceeds 650 vph for intersections with three approaches or 800 vph for intersections with four or more approaches.
- B. For one hour of an average day, the vehicular volumes fall above the appropriate curve of the graphs labeled as Figure 4C-4. Warrant 3, Peak Hour.

Table 17 presents the results for the analysis of Warrant 3 – Peak Hour Volume.

Table 17
Summary of Warrant 3 – Peak Hour Volume

Traffic Scenario	Study Year	No. of Plotted Points That Fall Above the Appropriate Line	Warrant Satisfied
SR-149 & Reco Drive			
2023 No-Build	2023	0	No
2023 Build	2023	0	No
2043 No-Build	2043	0	No
2043 Design Year	2043	3	Yes

11.5. Traffic Signal Warrant Study Summary

The following summary was generated based upon the findings in the Traffic Signal Warrant Study.

SR-149 & Reco Drive:

- Warrant 1 – Eight-Hour Vehicular Volume **is satisfied** at the intersection under the 2023 Build and 2043 Design Year Traffic Scenarios.
- Warrant 2 – Four-Hour Vehicular Volume **is satisfied** at the intersection under the 2023 Build and 2043 Design Year Traffic Scenarios.
- Warrant 3 – Peak Hour Vehicular Volume **is satisfied** at the intersection under the 2043 Design Year Traffic Scenarios.
- Based on Warrants 1, 2, and 3 being satisfied CESO recommends the installation of a traffic signal.

Detailed Signal Warrant Study calculations are located in Appendix I of the report.

12. Summary of Recommendations

12.1. Recommendations

The following summary of recommendations was generated based upon the findings in the Traffic Impact Study.

2023 No-Build Traffic Scenario (Responsibility – Others):

- No improvements are recommended or required.

2023 Build Traffic Scenario (Responsibility – Love’s):

SR-149 and Reco Drive:

- Signalize the intersection with an 80 second cycle length during the AM Peak Hour and 70 second cycle length during the PM Peak Hour. Coordinate this signal with the existing signals at the SR-149 & I-70 WB On/Off Ramps, SR-149 & I-70 EB On/Off Ramps, and SR-149 & Bond Drive intersections.
- Restripe the existing inside through lane on SR-149 to reflect a continuous two-way left-turn lane back to Pilot’s South Driveway.
- Construct a SBR turn lane to provide 200’ of storage plus a 50’ taper.

Reco Drive and Love’s Site Driveway #1

- Construct Love’s Site Driveway #1 to permit left-in, left-out, right-in, and right-out (full access) movements of passenger cars. Control this driveway with one (1) stop sign.

SR-149 and Love’s Site Driveway #2

- Construct Love’s Site Driveway #2 to permit left-in, left-out, right-in, and right-out (full access) movements of trucks and RVs. Control this driveway with one (1) stop sign.
- Construct a NBL turn lane to provide 175’ of storage plus a 50’ taper.
- Construct a SBR turn lane to provide 175’ of storage plus a 50’ taper.

2043 No-Build Traffic Scenario (Responsibility – Others):

- No improvements are recommended or required.

2043 Design Year Traffic Scenario (Responsibility – Love’s):

- No improvements are recommended or required.

Figure 12 illustrates the Love’s Development recommended improvements.



APPENDIX A
MEMORANDUM OF UNDERSTANDING BETWEEN
CESO & ODOT DISTRICT 11

MEMORANDUM OF UNDERSTANDING

TO: Jeremy Cessna, ODOT District 11 District Utility/Permit Supervisor
Chris Varcolla, ODOT District 11 Highway Safety District Coordinator

CC: Robert Matko, PE, PS, PTOE, CESO Engineering Manager
Kimberly Cooper, P.E., CESO Project Manager

FROM: Taylor Cline, P.E., CESO Lead Project Engineer

DATE: August 26, 2022

SUBJECT: Traffic Impact Study Scope
Love's Travel Stop Development
Southwest quadrant of the SR-149 and Reco Drive intersection
St. Clairsville, Ohio

The scope of services has been generated based on prior Traffic Study experience with ODOT District 11 and guidance from the most recent ODOT Traffic Impact Study (TIS) requirements. The site is located in the southwest corner of SR-149 and Reco Drive in Union Township, Belmont County, Ohio.

Traffic Study Scope – TIS Level 1 (follows ODOT requirements for TIS found in the State Highway Access Management Manual (SHAMM) Access Management Regulations

Level 1 traffic impact studies will require a full TIS as detailed in Chapter 9 of the SHAMM. According to section 9.41, a Level 1 TIS is required if the total number of trip ends (entering and exiting vehicles) ranges from 200-499.

According to the Ohio Traffic Forecasting Manual, Volume 1 (outline) and Volume 2 (procedures): Traffic Forecasting Background, the TIS must analyze two Build conditions (existing roadway and proposed roadway with background traffic plus site traffic) and No Build (existing roadway with background traffic) in the Design Year, which is Opening Year + 10 years for Level 1.

- 1. Conduct weekday (Tuesday – Thursday) peak hour (7:00 – 9:00 am and 4:00 – 6:00 pm) traffic counts at the following study intersections:**
 - **Traffic counts will be collected by our sub-consultant (Gewalt Hamilton Associates, Inc.) and will be video collected.**
 1. SR-149 & I-70 WB On/Off Ramps (Signal Controlled).
 2. SR-149 & I-70 EB On/Off Ramps (Signal Controlled).
 3. SR-149 & Reco Drive (Stop Sign Controlled).



2. **Inventory the existing roadway system (existing traffic controls, signage, and lane geometry).**
3. **Calculate the DHV's (Design Hourly Volumes).**
The 2022 Existing Traffic Volumes will be converted to Design Hourly Volumes (DHV's). DHV's will be calculated using factors determined using the date of traffic count and the Peak Hour to Design Hour Factor tables for the Ohio Department of Transportation, *Modeling and Forecasting Section*.
4. **Growth Rates**
Determine growth rates for each study roadway from historical traffic data obtained from ODOT TIMS.
This growth rate will be applied to the 2022 Design Hour Traffic Volumes to arrive at 2023 and 2033 No-Build Traffic Volumes.
5. **2023 No-Build Traffic Volumes**
Apply growth rate from #4 to the 2022 Existing DHV Traffic Volumes for one (1) year to arrive at 2023 No-Build Traffic Volumes.
6. **Prepare trip generation**
Prepare trip generation for the proposed development using the *Institute of Transportation Engineers Trip Generation Manual, 11th Edition*. CESO will look to use ITE Land Use Category 950 (Truck Stop), ITE Land Use Category 945 (Gasoline Service Station with Convenience Market), ITE Land Use Category 934 (Fast

Food Restaurant with Drive-Through Window), ITE Land Use Category 848 (Tire Store), and ITE Land Use Category 416 (Campground/Recreational Vehicle Park) for the proposed development.

Pass-By

Pass-By will be applied and will be based on percentages found in the *ITE Trip Generation Manual, 11th Edition*. In addition, diverted trips will be accounted for.

Internal Trip Reduction

Internal trip reduction will be applied and will be based on ITE procedures (NCHRP Report 684 Table 6-A and 6-P Internal Capture).

Note: Projections will be broken down into passenger cars and semi-truck traffic.

7. Determine directional distribution of development traffic

The directional distribution site traffic will be based on population and existing traffic patterns within the study area.

8. Assign project traffic to surrounding road network.

Based on the traffic projections, the development generated traffic volumes will be assigned to the adjacent street network.

9. 2023 Build Traffic Volumes

Add the 2023 No-Build Traffic Volumes to the Site Generated Traffic Volumes to arrive at 2023 Build Traffic Volumes.

10. 2033 No-Build Traffic Volumes

Apply growth rate from #4 to the 2022 Existing DHV Traffic Volumes for eleven (11) years to arrive at 2033 No-Build Traffic Volumes.

11. Perform capacity analysis (No-Build Traffic Scenario ~ 2033) at the key study intersections during the peak study hours.

Perform capacity analyses using procedures documented in the most recent edition of the *Highway Capacity Manual* and using Highway Capacity Software (HCS) Version 8.1 at the key study intersections utilizing 2033 No-Build Traffic Volumes during the study peak hour time periods.

Note: ODOT *Analysis and Traffic Simulation Manual (OATS)* format will be used for all capacity analysis.

15. 2033 Design Year Traffic Volumes

Add the 2033 No-Build Traffic Volumes to the Site Generated Traffic Volumes to arrive at 2033 Design Year Traffic Volumes.

16. Perform capacity analysis (Design Year Traffic Scenario ~ 2033) at the key study intersections and Site Driveway(s) during the peak study hours (Build Condition #1 per the updated SHAMM).

Perform capacity analyses using procedures documented in the most recent edition of the *Highway Capacity Manual* and using Highway Capacity Software (HCS) Version 8.1 at the key study intersections and site driveway(s) utilizing 2033 Design Year Traffic Volumes during the study peak hour time periods. Per the updated SHAMM, this analysis will analyze existing roadway geometry (i.e. number/types of

lanes) and existing traffic control (i.e. signalized/unsignalized) using the 2033 Design Year Traffic Volumes.

Note: The ODOT *OATS Manual* format will be used for all capacity analysis.

17. Perform capacity analysis (Design Year Traffic Scenario ~ 2033) at the key study intersections and Site Driveway(s) during the peak study hours (Build Condition #2 per the updated SHAMM).

Perform capacity analyses using procedures documented in the most recent edition of the *Highway Capacity Manual* and using Highway Capacity Software (HCS) Version 8.1 at the key study intersections and site driveway(s) utilizing 2033 Design Year Traffic Volumes during the study peak hour time periods (only if there is degradation when comparing Build Condition 1 to the 2033 No-Build Condition). Per the updated SHAMM, this analysis will analyze proposed roadway geometry (i.e. number/types of lanes) and proposed traffic control (i.e. signalized/unsignalized) using the 2033 Design Year Traffic Volumes.

Note: The ODOT *OATS Manual* format will be used for all capacity analysis.

18. Perform turn lane/queuing analysis.

Perform turn lane warrant/queuing analysis to determine if turn lanes or turn lane extensions are required at the study intersections. ODOT methodology (L&D Vol. 1, Figures 401-5a-5c and 401-6a-6d) will be used. ODOT L&D Vol. 1, Figure 401-9 and 401-10 will be used for turn lane storage lengths at the key study intersections and site driveway(s).

20. Based on Projected traffic volumes, recommend geometry for the proposed Love's Development Project.

Based on the projected volumes from the analysis, CESO will recommend the geometry for the proposed Love's Development including turn lane length calculations at the key study intersections and site driveway(s) per the L&D Vol. 1, Figure 401-9 and 401-10. A figure showing the conceptual geometry will be included.

21. Traffic Volume Submittal

CESO will submit all volumes including the trip generation summary for ODOT District 11 to review prior to commencing with the analysis portion of the study. The following information will be submitted for review and approval in a memorandum type format:

- Location map
- Site Plan
- Original raw traffic counts
- Growth rate documentation or calculations
- Existing Background Peak Hour Volume Plates (for each analysis period)
- Design Year Background (No-Build) Volume Plates (for each analysis period)
- Trip Generation worksheets or table
- Site Generated Volumes Plates (with Primary, Pass-by, and Diverted Link Trips delineated, for each analysis period)
- Design Year Final (Build) Volume Plates (for each analysis period)
- Documented assumptions including detailed sources and computations behind proprietary trip generation rates.

22. Prepare a written report summarizing the study process, conclusions, and recommendations.

Prepare a detailed report and submit to the ODOT District 11 office for review and approval.

Report shall include the following:

- a. Title Page
- b. Table of Contents including a list of figures, tables, and appendices.
- c. Introduction, which includes the description of the project, purpose of the report and executive summary.
- d. Proposed development description, including location, land use, and proposed use. This section will also include a regional map, vicinity map and site plan.
- e. Description of the study area.
- f. Existing conditions, including study site lane use, adjacent roadway description and traffic volumes. This section will also include a summary of existing traffic counts, graphic of existing daily and peak hour traffic and roadway condition diagram.
- g. Project traffic, including site traffic generation, distribution and assignment and non-site traffic for each time period to be analyzed. Graphics will be included showing the peak hour traffic volumes for each analysis time period and project phase for both the on and off-site traffic.
- h. Site traffic and total traffic volumes will be shown for each analysis time period.
- i. Traffic analysis showing tabular and graphic result of the analyses.
- j. Turn lane Analysis.
- k. Site Access Review.
- l. Summary of findings with conclusions and recommendations, including a graphic illustration of the recommendation/conclusion.
- m. Appendix including all computer-run data as well as any material related to the traffic study data collection and results.

CESO will submit all analysis files along with the written report.

If the above MOU is acceptable, please sign below. Thank you for your coordination and continued communication.

Jeremy Cessna, ODOT District 11, District Utility/Permit Supervisor

Date