



**CUY-90-14.90**

**PID 82119**

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**APPENDIX GE-09**

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**Soil Profile  
(Reference Document)**

State of Ohio  
Department of Transportation  
Jerry Wray, Director

**Innerbelt Bridge  
Construction Contract Group 2 (CCG2)**

**PROJECT DESCRIPTION**

AS PART OF THE CUY-90-14.90 PROJECT, IT IS PLANNED TO RECONFIGURE APPROXIMATELY 1.5 MILES OF EASTBOUND (STA. 1977+25.58 TO APPROPRIATE STA. 2058+50.69) IN CLEVELAND, CUYAHOGA COUNTY, OHIO. ASSOCIATED WORK INVOLVED IN THIS PROJECT IS THE CONSTRUCTION OF A NEW FIVE-LANE EASTBOUND BRIDGE STRUCTURE OVER THE CUYAHOGA RIVER VALLEY. ENTRANCE RAMPS AT WEST 14TH STREET AND EXIT RAMPS AT ONTARIO STREET AND EAST 9TH STREET WILL BE RECONFIGURED AND CONSTRUCTED. RECONFIGURATION OF EASTBOUND BRIDGES OVER STARKWEATHER AVENUE, KENILWORTH AVENUE, FAIRFIELD AVENUE, MAIN VIADUCT, ONTARIO STREET, AND EAST 9TH STREET ARE ALSO INCLUDED IN THIS PROJECT.

**HISTORIC RECORDS**

HISTORICAL INFORMATION WAS FOUND BY BBC&M ENGINEERING, INC. (BBCM) DURING CONSTRUCTION CONTRACT GROUP 1 (CCG1) IN THE GENERAL VICINITY OF THIS PROJECT. THEIR FINDINGS INCLUDE SEVERAL SUBSURFACE INVESTIGATIONS PERFORMED IN THE GENERAL PROJECT VICINITY AS EARLY AS 1923. AVAILABLE INFORMATION WITHIN THE PROJECT LIMITS OBTAINED FROM THE HISTORICAL RECORDS SEARCH IS LISTED AS FOLLOWS:

\* SOIL PROFILES WERE AVAILABLE FOR THE 1923 INVESTIGATION PERFORMED BY THE CLEVELAND UNION TERMINAL COMPANY FOR THE CUYAHOGA VIADUCT BRIDGE, WHICH IS NOW OWNED AND OPERATED BY THE GREATER CLEVELAND REGIONAL TRANSIT AUTHORITY.

\* MULTIPLE SUBSURFACE INVESTIGATIONS WERE CONDUCTED BY ODOT IN 1958, 1963, 1964, 1990 AND 1992. THE PRE-1990 ODOT INVESTIGATIONS WERE GENERALLY PERFORMED FOR THE PLANNED INNERBELT FREEWAY AND BRIDGE. THE 1990 AND 1992 ODOT BORINGS WERE PERFORMED TO INVESTIGATE THE OBSERVED SLOPE MOVEMENTS IN THE VICINITY OF THE WEST END PIER AND PIER 1 OF THE EXISTING INNERBELT BRIDGE.

\* HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEERS (HNTB) PERFORMED INVESTIGATIONS BETWEEN 1954 AND 1956 PRIMARILY AT THE PROPOSED ABUTMENT AND PIER LOCATIONS FOR THE EXISTING INNERBELT BRIDGE.

\* SOIL PROFILES WERE AVAILABLE FOR THE 1955 INVESTIGATION BY NEW YORK, CHICAGO AND ST. LOUIS RAILROAD LINE (NOW OWNED BY NORFOLK SOUTHERN CORPORATION) OVER THE CUYAHOGA RIVER LOCATED IMMEDIATELY TO THE NORTH OF THE EXISTING INNERBELT BRIDGE.

\* IN 1986, BORINGS WERE PERFORMED BY THE DAVID V. LEWIN CORPORATION (LEWIN) AT THE SITE OF A PROPOSED DOMED STADIUM. THE SITE IS NOW OCCUPIED BY PROGRESSIVE FIELD.

\* IN 1986 AND 1987, R&R INTERNATIONAL, INC. (R&R) PERFORMED BORINGS FOR THE EXISTING ABBEY AVENUE VIADUCT OVER THE EXISTING NORFOLD SOUTHERN RAILROAD AND SCRANTON ROAD.

\* IN 2000 AND 2001, DLZ CORPORATION (DLZ) PERFORMED THREE (3) BORINGS ADJACENT TO THE EXISTING INNERBELT BRIDGE AS A PART OF A STABILITY EVALUATION FOR THE CUY-90-15.24 PROJECT.

\* BBCM PERFORMED SUBSURFACE INVESTIGATIONS BETWEEN 1994 AND 2006 GENERALLY TO INVESTIGATE VARIOUS ASPECTS OF THE CUY-90-15.24 AND CUY-90-14.52 PROJECTS. THE SUBSURFACE INVESTIGATIONS ALSO INCLUDED INSTALLATION OF INSTRUMENTATION SUCH AS INCLINOMETERS AND PIEZOMETERS. THE PURPOSE OF THE BBCM INVESTIGATIONS IN THE 1990'S WAS TO MONITOR THE SLOPE MOVEMENTS AT THE WEST END PIER AND PIER 1 OF THE EXISTING INNERBELT BRIDGE AND AID THE DESIGN OF A STABILIZATION STRUCTURE WAS COMPLETED IN 1999. AN ADDITIONAL SUBSURFACE INVESTIGATION AT THE WEST BANK OF THE CUYAHOGA RIVER WAS PERFORMED BY BBCM IN 2006 TO EVALUATE THE LIMITS OF INSTABILITY IN THE WEST BANK SLOPE AND EVALUATE VARIOUS ALTERNATIVES FOR STABILIZING THE SLOPE IN SUPPORT OF THE FOUNDATIONS IN THIS AREA FOR THE PROPOSED NEW BRIDGE (WESTBOUND I-90 BRIDGE) TO BE LOCATED JUST NORTH OF THE EXISTING INNERBELT BRIDGE. OHIO UNIVERSITY PERFORMED SEVERAL CPT TESTS IN CONJUNCTION WITH BBCM'S INVESTIGATIONS.

\* BBCM PERFORMED A PRELIMINARY SUBSURFACE INVESTIGATION IN 2006 FOR THE PROPOSED WESTBOUND I-90 BRIDGE ALIGNMENT. IN ADDITION TO THE BORINGS PERFORMED FOR THE PROPOSED WESTBOUND I-90 BRIDGE, BORINGS WERE PERFORMED FOR THE FOLLOWING: EXISTING ROADWAYS TO BE REHABILITATED OR RECONFIGURED; NEW ROADWAYS; NEW, REHABILITATED AND/OR RECONFIGURED INTERCHANGE RAMPS; THE I-90 MAINLINE; AND, ASSOCIATED NEW AND REHABILITATED STRUCTURES (I.E. BRIDGES AND RETAINING WALLS).

\* BARR & PREVOST DRILLED 80 BORINGS BETWEEN MAY AND OCTOBER OF 2009 AND BBCM DRILLED 7 BORINGS BETWEEN JUNE AND SEPTEMBER OF 2009 TO SUPPLEMENT THE SUBSURFACE INVESTIGATIONS PERFORMED BY BBCM BETWEEN 1994 AND 2006 BASED ON THE MOST RECENT PLANS FOR THE CUY-90-14.90 PROJECT. ADDITIONALLY, ODOT PERFORMED 2 CPT TESTS ON AUGUST 18, 2009.

\* B&P PERFORMED AN ADDITIONAL 6 BORINGS BETWEEN APRIL AND MAY OF 2010 AT LOCATIONS REQUESTED BY THE DESIGN BUILD TEAM FINALISTS.

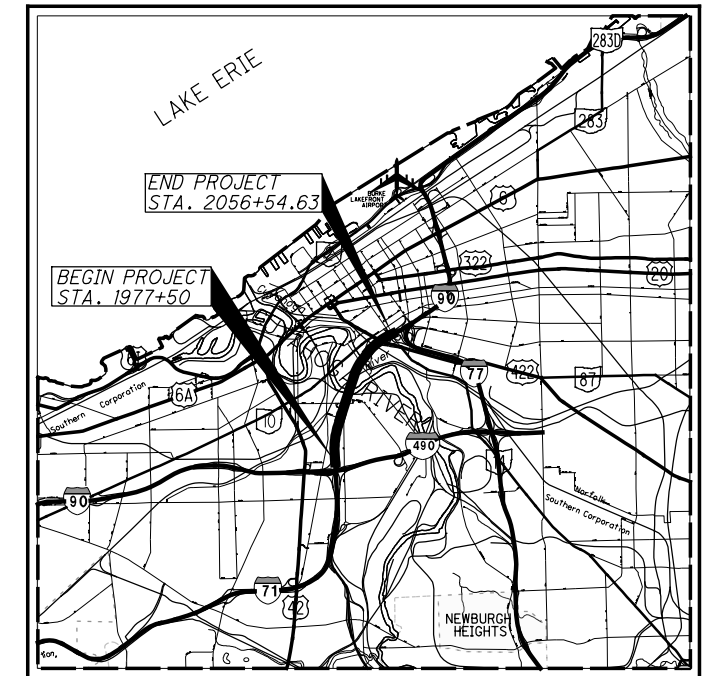
ALL AVAILABLE SOIL INFORMATION THAT CAN BE CONVENIENTLY SHOWN ON THE SOIL PROFILE SHEETS HAVE BEEN SO REPORTED.

- CONTINUE ON SHEET 2 -

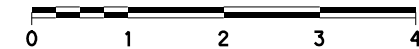
NOTES: BORINGS LOGS ON INFORMATION AVAILABLE AT THE TIME OF SUBMITTAL ARE SUBMITTED IN A BINDER UNDER SEPARATE COVER.

**LEGEND**

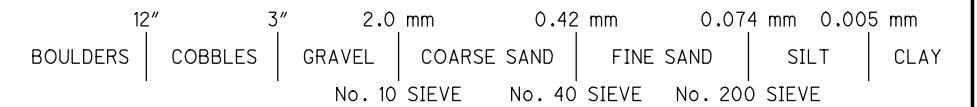
DESCRIPTION	ODOT CLASS	CLASSIFIED MECH./VISUAL	
GRAVEL AND/OR STONE FRAGS.	A-1-a	1	10
GRAVEL AND/OR STONE FRAGS. WITH SAND	A-1-b	75	67
GRAVEL AND/OR STONE FRAGS. WITH SAND AND SILT	A-2-4	10	8
FINE SAND	A-3	22	23
COARSE AND FINE SAND	A-3a	117	173
SANDY SILT	A-4a	63	31
SILT	A-4b	136	69
ELASTIC SILT AND CLAY	A-5	1	0
SILT AND CLAY	A-6a	142	90
SILTY CLAY	A-6b	30	27
ELASTIC CLAY	A-7-5	3	5
CLAY	A-7-6	5	3
ORGANIC CLAY	A-8b	1	0
	<b>TOTAL</b>	<b>606</b>	<b>506</b>
UNCONTROLLED FILL	VISUAL		
BOULDERY ZONE	VISUAL		
SHALE	VISUAL		
PAVEMENT OR BASE = X = APPROXIMATE THICKNESS	VISUAL		
SOD AND TOPSOIL = X = APPROXIMATE THICKNESS	VISUAL		
BORING LOCATION - PLAN VIEW			
HISTORIC BORING LOCATION - PLAN VIEW			
DRIVE SAMPLE AND/OR ROCK CORE BORING PLOTTED TO VERTICAL SCALE ONLY. HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPHY.			
AUGER BORING PLOTTED TO VERTICAL SCALE ONLY. HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPHY.			
N60 INDICATES STANDARD PENETRATION RESISTANCE NORMALIZED TO 60% DRILL ROD ENERGY RATIO.			
WC INDICATES WATER CONTENT IN PERCENT.			
W INDICATES FREE WATER ELEVATION.			
INDICATES STATIC WATER ELEVATION.			
INDICATES A PLASTIC MATERIAL WITH A MOISTURE CONTENT EQUAL TO OR GREATER THAN THE LIQUID LIMIT MINUS 3.			
INDICATES A NON-PLASTIC MATERIAL WITH A MOISTURE CONTENT GREATER THAN 25% OR GREATER THAN 19% WITH A WET APPEARANCE.			
NP INDICATES A NON-PLASTIC SAMPLE.			
SS INDICATES A SPLIT SPOON SAMPLE, STANDARD PENETRATION TEST.			
ST INDICATES A SHELBY TUBE SAMPLE.			
TR INDICATES TOP OF ROCK.			



LOCATION MAP  
SCALE IN MILES



**PARTICLE SIZE DEFINITIONS**



**INDEX OF SHEETS**

LOCATION FROM STA. TO STA.	PLAN VIEW SHEET	PROFILE SHEET
CL IR 90 EB		
1977+25 1988+50	21	22
1988+50 2001+00	23	24, 25, 51
2001+00 2013+50	26	27, 28, 29, 30, 45, 46, 47, 52, 53, 54, 55
2013+50 2026+00	31	32, 33, 56, 57, 58, 59, 60
2026+00 2038+50	34	35, 36, 61, 62, 63, 64
2038+50 2051+00	37	38, 39, 40, 48, 49, 65, 66
2051+00 2063+50	41	42, 49, 50, 67
2063+50 2067+00	43	44
IR RAMP B2		
200+00 207+00	26	45
IR E. 14TH ST. NB		
0+00 8+00	26	46
IR RAMP B3		
5340+00 5362+00	37, 41	48, 49
IR E. 9TH ST.		
31+00 44+00	41	50

DRILLING - RW 06/13/11-08/22/11, 03/14-15/2012  
 DRAWN - CMH 08/01/11-10/21/11, 04/10-14/2012  
 REVIEWED - JEP 11/02/2011, 04/16/2012



GEOLOGY

THE SITE IS LOCATED ON THE ERIE LAKE PLAIN PHYSIOGRAPHIC PROVINCE IN NORTHERN OHIO, NEAR THE SHORELINE OF LAKE ERIE. THIS PROVINCE CONSISTS OF THE EDGE OF AN ICE-AGE LAKE BASIN WITH PLEISTOCENE-AGE LACUSTRINE SAND, SILT, CLAY, AND WAVE-PLANED TILL OVER DEVONIAN AND MISSISSIPPIAN-AGE SHALES AND SANDSTONES.

EXPLORATION FINDINGS

IN GENERAL, THE BORINGS ENCOUNTERED SOIL CONDITIONS CONSISTENT WITH THE GEOLOGICAL MODEL FOR THE SITE.

THE MAJORITY OF THE PROJECT LIMITS LIE WITHIN THE PRESENT CUYAHOGA RIVER VALLEY. A LARGE PORTION OF THE CUYAHOGA RIVER VALLEY IS CLASSIFIED AS "MADE LAND" WITH URBAN COVER COMPOSED OF FILL MATERIALS OF VARIABLE COMPOSITION AND DEPTH. BENEATH THE FILL MATERIALS, APPROXIMATELY 20-50 FEET OF ALLUVIUM DEPOSITS OF HOLOCENE AGE CONSISTING PREDOMINATELY OF VERY-LOOSE TO MEDIUM-DENSE SANDS AND SILTS AND VERY SOFT TO MEDIUM STIFF AND ORGANIC CLAY IN SOME LOCATIONS WERE GENERALLY ENCOUNTERED. THE ALLUVIUM MATERIALS WERE DEPOSITED BY PRECURSOR TO THE CUYAHOGA RIVER. FOLLOWING THE ALLUVIUM DEPOSITS ARE APPROXIMATELY 55 TO 100 FEET OF LACUSTRINE DEPOSITS CONSISTING PRIMARILY OF MEDIUM-STIFF TO VERY STIFF SILTY CLAYS. THESE LACUSTRINE SOILS WERE DEPOSITED BY A SERIES OF LARGE PROGLACIAL LAKES WHICH COVERED THE GREATER CLEVELAND AREA NEAR THE END OF THE WISCONSINAN GLACIAL PERIOD. MANY SLOPE FAILURES HAVE BEEN OBSERVED OVER THE YEARS ON THE SIDE SLOPES ADJACENT TO THE CUYAHOGA RIVER. THEY HAVE OCCURRED PRIMARILY WITHIN THESE LACUSTRINE DEPOSITS. BELOW THE LACUSTRINE DEPOSITS, APPROXIMATELY 25 TO 65 FEET OF WISCONSINAN OR ILLINOIAN GLACIAL TILL WERE ENCOUNTERED. THE TILL PRIMARILY CONSISTS OF DENSE TO VERY-DENSE SANDY SILT OR VERY STIFF TO HARD CLAYEY SILT. THE TILL OVERLIES OHIO DEVONIAN SHALE WHICH WAS ENCOUNTERED AT AN ELEVATION OF 398 TO 455 FT MEAN SEA LEVEL.

SUBSURFACE EXPLORATION

B&P PERFORMED A SUBSURFACE INVESTIGATION FOR CUY-90-14.90 PROJECT BETWEEN JUNE AND AUGUST OF 2011. IT INCLUDES 11 ROADWAY BORINGS AT VARIOUS LOCATIONS, 2 STRUCTURES BORINGS AT BRIDGE I-90 OVER STARKWEATHER AVENUE, 2 STRUCTURE BORINGS AT BRIDGE I-90 OVER KENILWORTH AVENUE, 2 STRUCTURE BORINGS AT BRIDGE I-90 OVER FAIRFIELD AVENUE, 18 STRUCTURE BORINGS AT MAIN VIADUCT BRIDGE I-90 OVER CUYAHOGA RIVER AND RAMP OVER ABBEY, 9 RETAINING WALL BORINGS, 3 STRUCTURE BORINGS AT BRIDGE I-90 OVER ONTARIO STREET, AND 3 STRUCTURE BORINGS AT BRIDGE I-90 OVER EAST 9TH STREET.

ADDITIONAL FIVE BORINGS WITH DEPTHS OF 25 FEET WERE DRILLED FOR HIGH MAST LIGHTING TOWERS ON MARCH 14 AND 15, 2012. THEY INCLUDE 1 BORING CLOSE TO THE BRIDGE I-90 OVER FAIRFIELD AVENUE, 4 BORINGS IN EAST END ROADWAY.

THE BORINGS WERE DRILLED WITH A TRUCK MOUNTED MOBILE B-61 ORV RIG, USING 3/4 -INCH I.D. HOLLOW STEM AUGERS TO ADVANCE THE HOLES THROUGH SOIL. THE TRICONE RIG WAS ALSO USED IN HARD DRILLING CONDITIONS. MOST DISTURBED SOIL SAMPLES WERE OBTAINED IN ACCORDANCE WITH THE STANDARD PENETRATION TEST (AASHTO T206) AT 2.5-FOOT AND 5.0-FOOT INTERVALS AND PLACED IN SEALED JARS. SOME OF THE DISTURBED SOIL SAMPLES WERE OBTAINED AT 10-FOOT INTERVALS AT SOIL BELOW 50 FEET. UNDISTURBED SOIL SAMPLES WERE OBTAINED AT THE DEPTHS SHOWN ON THE LOGS AND IN THE PROFILE, IN ACCORDANCE WITH AASHTO T207. WHERE ENCOUNTERED, BEDROCK WAS CORED USING AN NQ ROCK CORE BARREL WITH WATER AS A CIRCULATING/COOLING FLUID. RETRIEVED ROCK CORE SAMPLES WERE STORED IN COMPARTMENTAL CORE BOXES. UPON COMPLETION, THE DEPTH TO ANY ACCUMULATED GROUNDWATER WAS MEASURED, THE BORINGS WERE BACKFILLED OR SEALED IN ACCORDANCE WITH ODOT REQUIREMENTS, AND THE SURFACE OF THE EXISTING PAVEMENT AT THE BORING LOCATIONS WAS REPAIRED USING COLD-PATCH ASPHALT OR CONCRETE, WHERE APPROPRIATE.

THE LOGS OF BORINGS FOR THESE 2011 SUBSURFACE INVESTIGATIONS HAVE ALSO INCLUDED SEPARATELY IN THE AFORMENTIONED BINDER OF LOGS.

SPECIFICATIONS

THIS GEOTECHNICAL EXPLORATION WAS PERFORMED IN ACCORDANCE WITH THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, OFFICE OF GEOTECHNICAL ENGINEERING, SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS (WITH REVISION THROUGH JANUARY 2011).

AVAILABLE INFORMATION

ALL AVAILABLE SOIL AND BEDROCK INFORMATION THAT CAN BE CONVENIENTLY SHOWN ON THE SOIL PROFILE SHEETS HAS BEEN SO REPORTED. ADDITIONAL SUBSURFACE EXPLORATIONS MAY HAVE BEEN MADE TO STUDY SOME SPECIAL ASPECT OF THE PROJECT. COPIES OF THIS DATA, IF ANY, MAY BE INSPECTED IN THE DISTRICT DEPUTY DIRECTOR'S OFFICE, THE OFFICE OF GEOTECHNICAL ENGINEERING AT 1600 WEST BROAD STREET OR THE OFFICE OF STRUCTURAL ENGINEERING AT 1980 WEST BROAD STREET.

SUMMARY OF SOIL TEST DATA

STATION & OFFSET	FROM TO	SAMPLE ID	% REC	IR 90 EB						LL	PL	PI	% WC	ODOT CLASS
				% GR	% CS	% FS	% SILT	% CLAY						
B-001-0-11 STA. 1977+48.9, 91.1' RT NORTHING = 660332.480 EASTING = 2189843.135	01.00-02.50	SS-1	89	17	29	33	16	5	NP	NP	NP	8	A-3a (0)	
	03.50-05.00	SS-2	83			SAME AS SS-3						10	A-4a (V)	
	06.00-07.50	SS-3	83	5	14	39	34	8	NP	NP	NP	12	A-4a (I)	
B-002-0-11 STA. 1979+17.5, 20.5' RT NORTHING = 660496.338 EASTING = 2189762.245	01.00-02.50	SS-1	78	19	25	35	14	7	NP	NP	NP	9	A-3a (0)	
	03.50-05.00	SS-2	67	23	23	39	12	3	NP	NP	NP	16	A-3a (0)	
	06.00-07.50	SS-3	100			SAME AS SS-2						6	A-3a (V)	
B-005-0-11 STA. 1983+49.4, 41.0' RT NORTHING = 660925.716 EASTING = 2189769.528	01.00-02.50	SS-1	78	14	25	41	14	6	NP	NP	NP	7	A-3a (0)	
	03.50-05.00	SS-2	94	19	20	44	11	6	NP	NP	NP	7	A-3a (0)	
	06.00-07.50	SS-3	100			SAME AS SS-2						8	A-3a (V)	
B-006-0-11 STA. 1987+23.8, 32.6' RT NORTHING = 661296.045 EASTING = 2189787.055	01.00-02.50	SS-1	100	23	29	24	17	7	NP	NP	NP	7	A-1-b (0)	
	03.50-05.00	SS-2	94			SAME AS SS-3						10	A-3a (V)	
	06.00-07.50	SS-3	100	13	20	40	19	8	NP	NP	NP	8	A-3a (0)	
B-009-0-11 STA. 1991+67.5, 30.1' RT NORTHING = 661733.053 EASTING = 2189853.956	01.00-02.50	SS-1	78	15	33	32	14	6	NP	NP	NP	8	A-3a (0)	
	03.50-05.00	SS-2	100			SAME AS SS-1						7	A-3a (V)	
	06.00-07.50	SS-3	89			SAME AS SS-1						9	A-3a (V)	
B-010-0-11 STA. 1997+40.7, 41.3' LT NORTHING = 662310.127 EASTING = 2189879.361	01.00-02.50	SS-1	72	12	24	45	16	3	NP	NP	NP	9	A-3a (0)	
	03.50-05.00	SS-2	100	3	19	51	18	9	NP	NP	NP	10	A-3a (0)	
	06.00-07.50	SS-3	83			SAME AS SS-2						10	A-3a (V)	
B-032-2-11 STA. 2051+05.4, 225.8' RT NORTHING = 666666.460 EASTING = 2192496.732	01.50-03.00	SS-1	89	44	23	21	7	5	NP	NP	NP	14	A-1-b (0)	
	03.50-05.00	SS-2	89	30	27	29	8	6	NP	NP	NP	12	A-1-b (0)	
	06.00-07.50	SS-3	78			SAME AS SS-2						10	A-1-b (V)	
B-037-0-11 STA. 2055+50.6, 221.4' LT NORTHING = 667251.358 EASTING = 2192715.250	01.00-02.50	SS-1	28	30	21	28	14	7	NP	NP	NP	3	A-1-b (0)	
	03.50-05.00	SS-2	83	16	27	31	21	5	NP	NP	NP	8	A-3a (0)	
	06.00-07.50	SS-3	89			SAME AS SS-2						7	A-3a (V)	
B-038-1-11 STA. 2056+21.8, 102.8' RT NORTHING = 666979.725 EASTING = 2192905.956	01.00-02.50	SS-1	22			SAME AS B-032-2-11 SS-1						3	A-1-b (V)	
	03.50-05.00	SS-2	56	34	15	29	17	5	NP	NP	NP	3	A-2-4 (0)	
	06.00-07.50	SS-3	39	32	9	43	12	4	NP	NP	NP	5	A-3a (0)	
B-041-0-11 (ALIGNMENT: EX. IR 90) STA. 67+80.3, 110.8' LT NORTHING = 667499.564 EASTING = 2193443.797	01.00-02.50	SS-1	94	17	34	27	13	9	NP	NP	NP	10	A-1-b (0)	
	03.50-04.50	SS-2	83	20	37	32	7	4	NP	NP	NP	14	A-1-b (0)	
	04.50-05.00	SS-2	83			SAME AS B-039-0-11 SS-25						19	A-6a (V)	
	06.00-07.50	SS-3	100			SAME AS B-038-1-11 SS-3						7	A-3a (V)	
B-042-0-11 (ALIGNMENT: EX. IR 90) STA. 70+73.1, 45.2' LT NORTHING = 667596.495 EASTING = 205850.687	01.00-02.50	SS-1	100	23	4	52	16	5	NP	NP	NP	8	A-3a (0)	
	03.50-05.00	SS-2	78	8	12	56	18	6	NP	NP	NP	8	A-3a (0)	
	06.00-07.50	SS-3	78			SAME AS SS-2						7	A-3a (V)	



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-1, Depth: 159.2 - 159.7 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

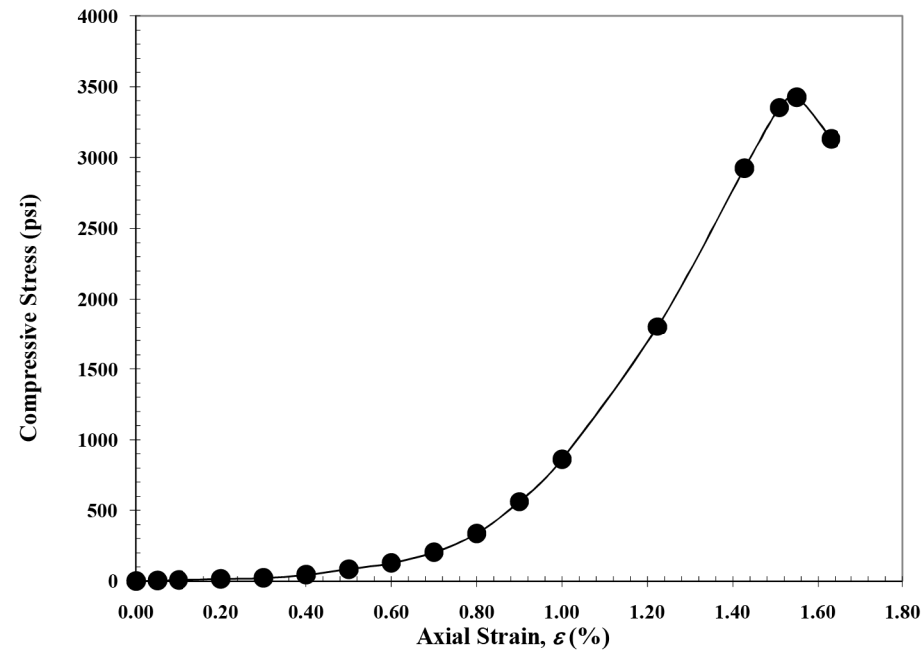
Average Dia., $D_{avg}$ (in):	1.97
Average Height $H_{avg}$ (in):	4.90
Area, $A$ (in <sup>2</sup> ):	3.06
Volume, $V$ (in <sup>3</sup> ):	15.02
Wet Mass of Specimen (lb):	1.4
Moisture Content (%):	2.9
Dry Mass of Specimen (lb):	1.4
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	160.0
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	155.6

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 3425  
Strain (%): 1.6



**NOTES:** Slightly strong, dark gray to black, SHALE.

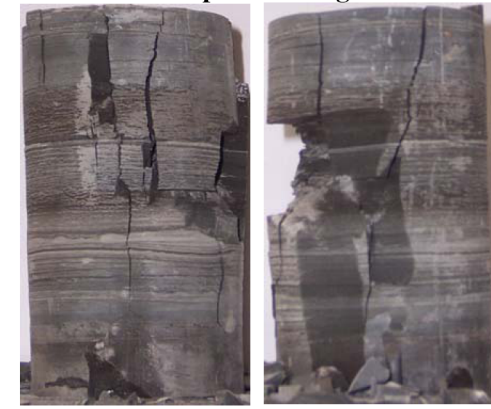
**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-2, Depth: 165.1 - 165.5 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

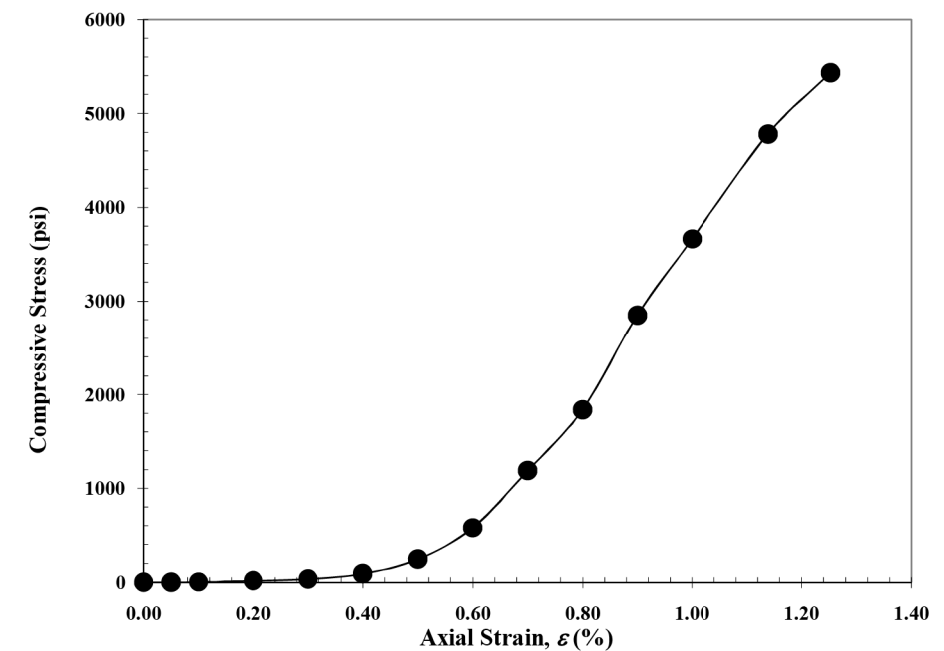
Average Dia., $D_{avg}$ (in):	1.97
Average Height $H_{avg}$ (in):	4.39
Area, $A$ (in <sup>2</sup> ):	3.06
Volume, $V$ (in <sup>3</sup> ):	13.42
Wet Mass of Specimen (lb):	1.2
Moisture Content (%):	2.5
Dry Mass of Specimen (lb):	1.2
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	160.8
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	156.8

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 5431  
Strain (%): 1.3



**NOTES:** Moderately strong, dark gray, SHALE.



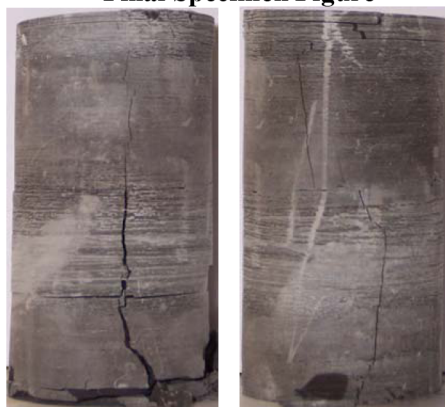
**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-3, Depth: 172.9 - 173.3 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

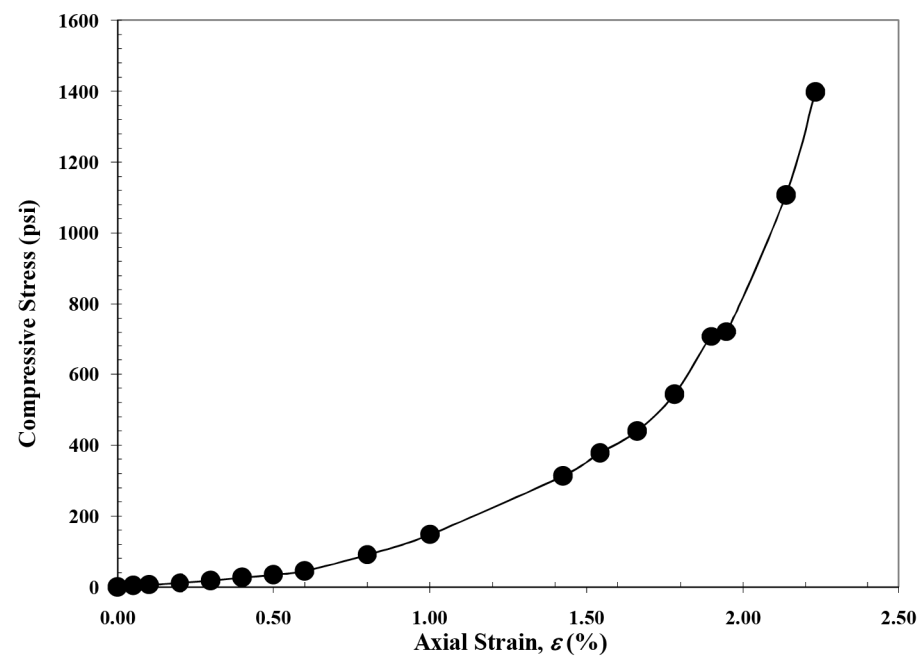
Average Dia., $D_{avg}$ (in):	1.98
Average Height $H_{avg}$ (in):	4.21
Area, $A$ (in <sup>2</sup> ):	3.07
Volume, $V$ (in <sup>3</sup> ):	12.94
Wet Mass of Specimen (lb):	1.2
Moisture Content (%):	2.5
Dry Mass of Specimen (lb):	1.2
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	159.6
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	155.8

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 1398  
Strain (%): 2.2



**NOTES:** Weak, dark gray, SHALE.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-4, Depth: 174.0 - 174.4 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

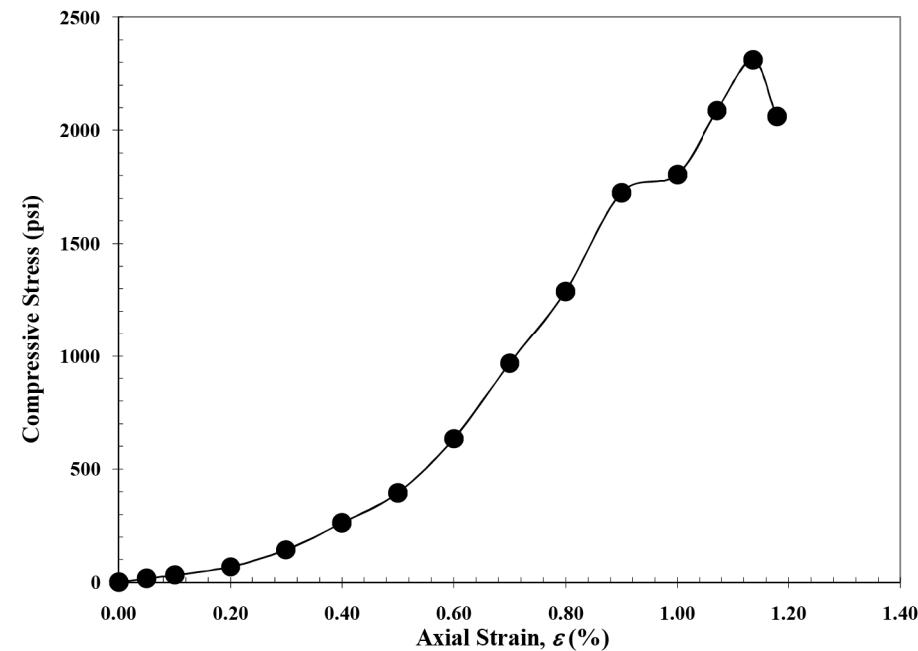
Average Dia., $D_{avg}$ (in):	1.98
Average Height $H_{avg}$ (in):	4.66
Area, $A$ (in <sup>2</sup> ):	3.07
Volume, $V$ (in <sup>3</sup> ):	14.30
Wet Mass of Specimen (lb):	1.3
Moisture Content (%):	2.6
Dry Mass of Specimen (lb):	1.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	158.7
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	154.7

**Final Specimen Figure**

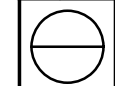


**Results**

Unconfined Compressive Strength (psi): 2310  
Strain (%): 1.1



**NOTES:** Slightly strong, gray to dark gray, SHALE.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-5, Depth: 181.6 - 182.0 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

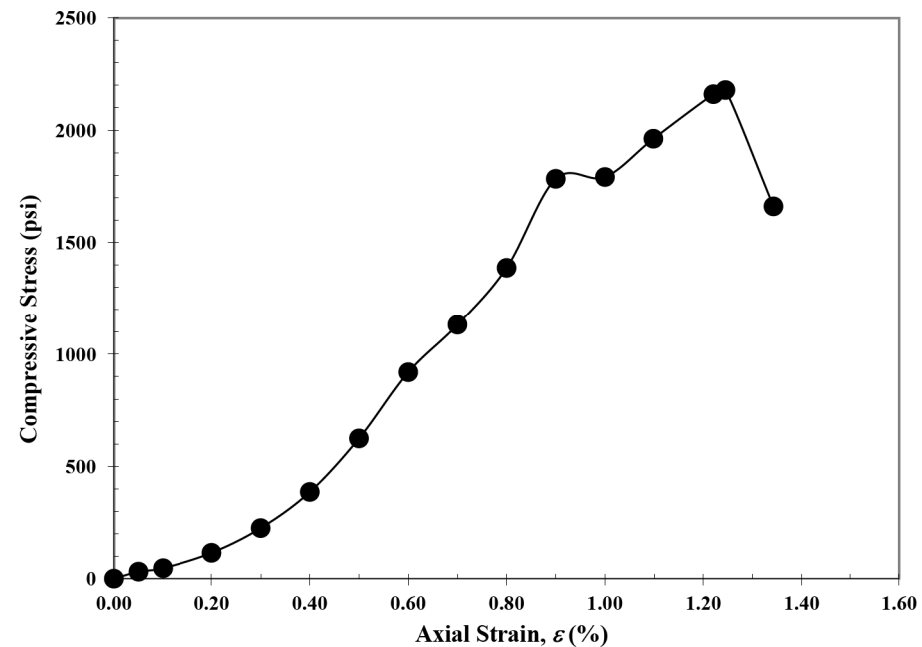
Average Dia., $D_{avg}$ (in):	1.98
Average Height, $H_{avg}$ (in):	4.10
Area, $A$ (in <sup>2</sup> ):	3.09
Volume, $V$ (in <sup>3</sup> ):	12.65
Wet Mass of Specimen (lb):	1.2
Moisture Content (%):	2.4
Dry Mass of Specimen (lb):	1.1
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	159.2
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	155.4

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 2179  
Strain (%): 1.3



**NOTES:** Slightly strong, gray to dark gray, SHALE.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-6, Depth: 186.0 - 186.5 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

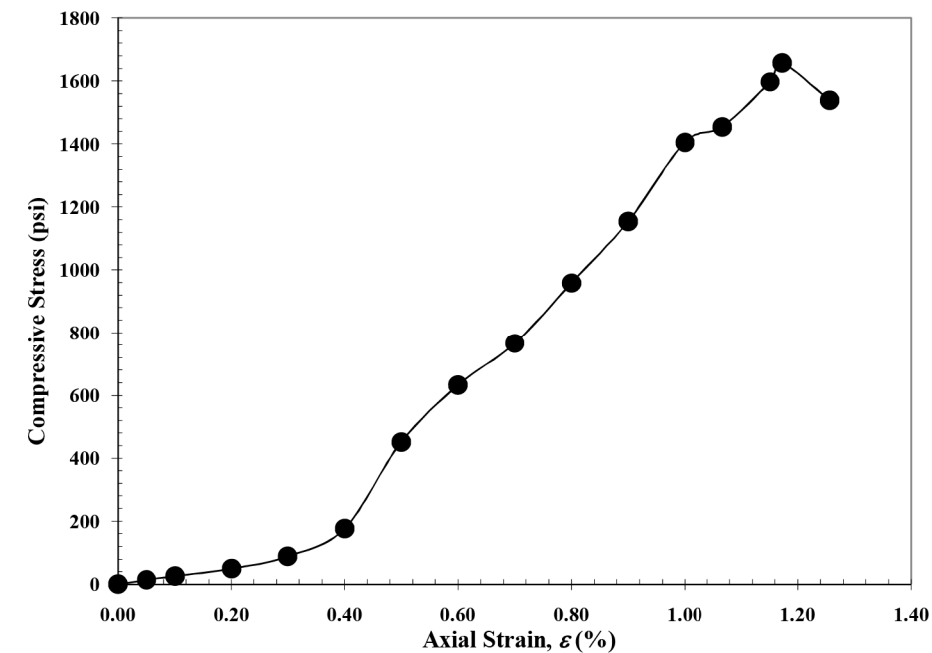
Average Dia., $D_{avg}$ (in):	1.97
Average Height, $H_{avg}$ (in):	4.78
Area, $A$ (in <sup>2</sup> ):	3.06
Volume, $V$ (in <sup>3</sup> ):	14.62
Wet Mass of Specimen (lb):	1.4
Moisture Content (%):	2.7
Dry Mass of Specimen (lb):	1.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	163.9
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	159.5

**Final Specimen Figure**

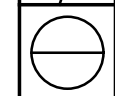


**Results**

Unconfined Compressive Strength (psi): 1656  
Strain (%): 1.2



**NOTES:** Slightly strong, gray, SHALE.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-7, Depth: 189.3 - 189.7 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

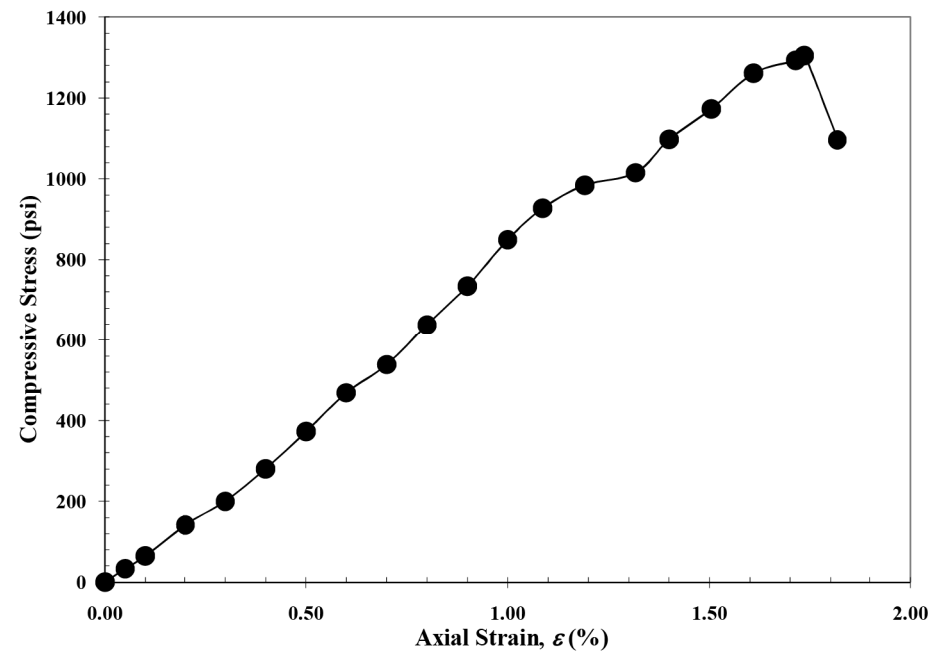
Average Dia., $D_{avg}$ (in):	1.97
Average Height $H_{avg}$ (in):	4.78
Area, $A$ (in <sup>2</sup> ):	3.05
Volume, $V$ (in <sup>3</sup> ):	14.61
Wet Mass of Specimen (lb):	1.4
Moisture Content (%):	3.0
Dry Mass of Specimen (lb):	1.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	162.2
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	157.5

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 1305  
Strain (%): 1.7



**NOTES:** Weak, darkgray, SHALE.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-8, Depth: 194.8 - 195.1 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

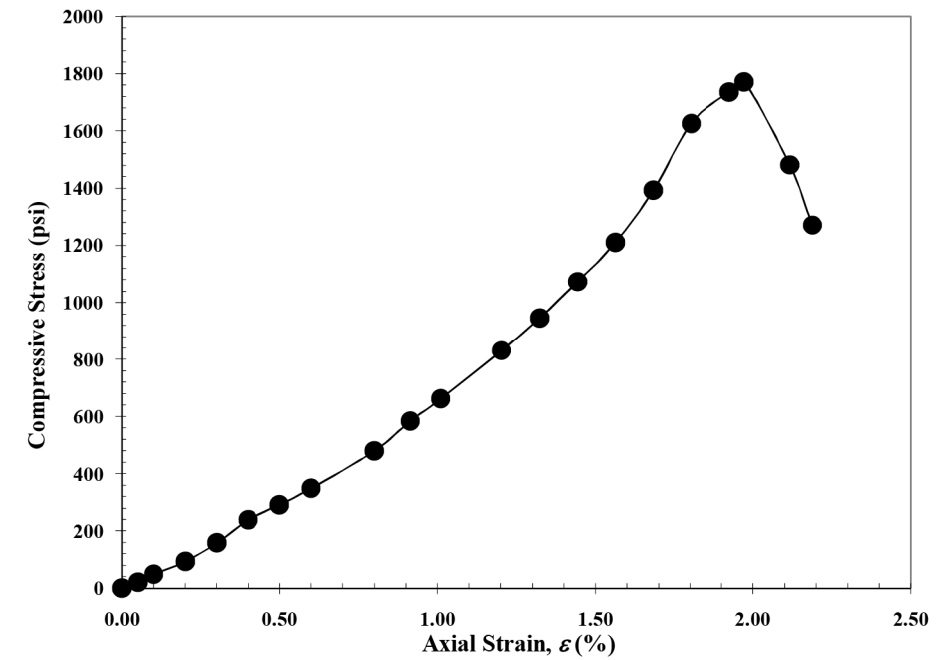
Average Dia., $D_{avg}$ (in):	1.98
Average Height $H_{avg}$ (in):	4.16
Area, $A$ (in <sup>2</sup> ):	3.07
Volume, $V$ (in <sup>3</sup> ):	12.74
Wet Mass of Specimen (lb):	1.2
Moisture Content (%):	3.0
Dry Mass of Specimen (lb):	1.2
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	161.5
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	156.8

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 1771  
Strain (%): 2.0



**NOTES:** Slightly strong, dark gray, SHALE.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-9, Depth: 201.4 - 201.8 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

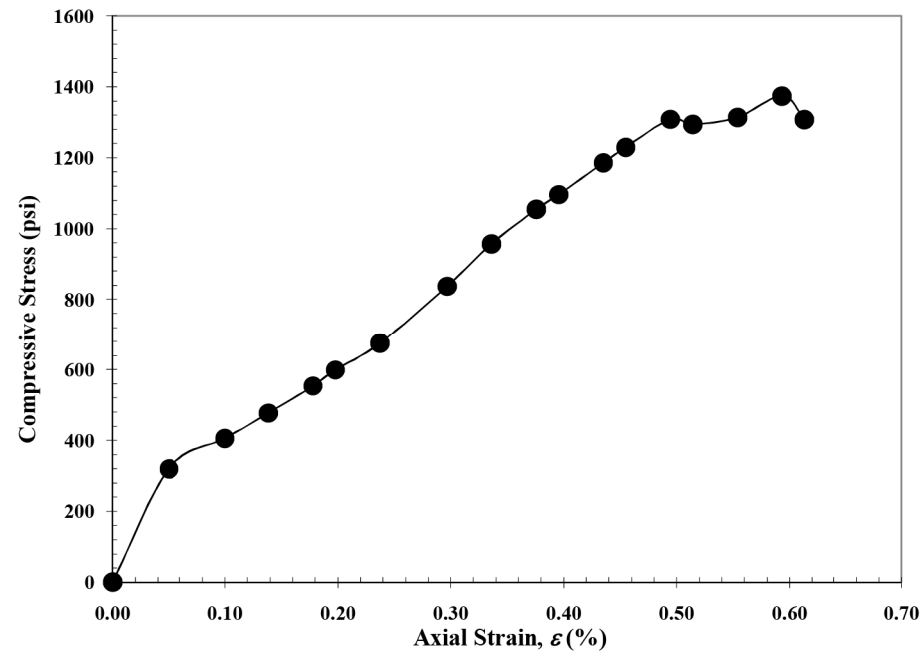
Average Dia., $D_{avg}$ (in):	1.97
Average Height $H_{avg}$ (in):	5.06
Area, $A$ (in <sup>2</sup> ):	3.06
Volume, $V$ (in <sup>3</sup> ):	15.45
Wet Mass of Specimen (lb):	1.5
Moisture Content (%):	2.4
Dry Mass of Specimen (lb):	1.4
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	162.4
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	158.6

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 1374  
Strain (%): 0.6



**NOTES:** Weak, dark gray, SHALE.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-017-0-11, R-10, Depth: 207.6 - 207.9 ft)  
Tested Date: 6/23/2011

**Specimen Properties**

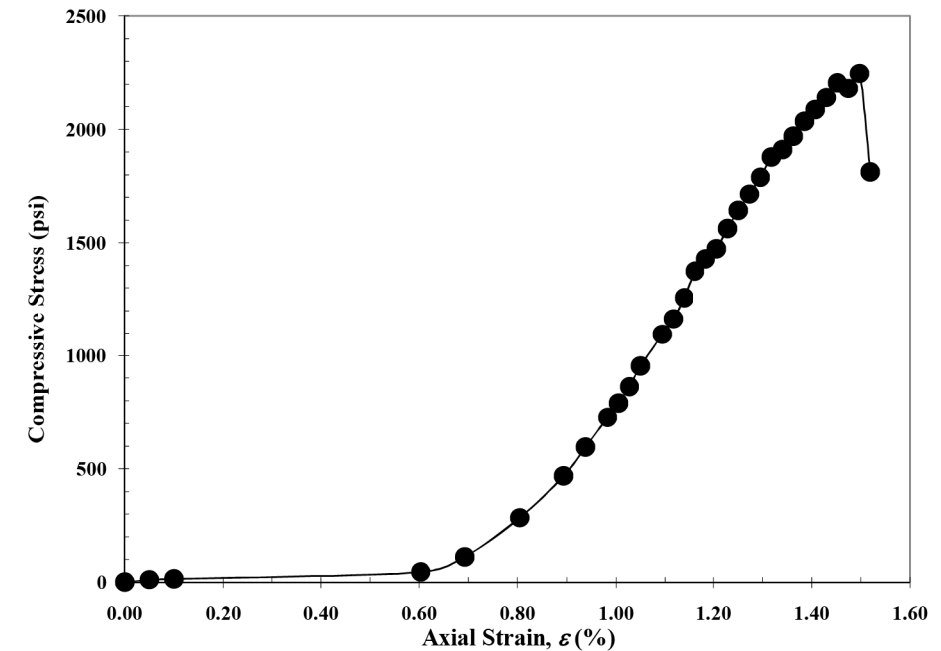
Average Dia., $D_{avg}$ (in):	1.98
Average Height $H_{avg}$ (in):	4.48
Area, $A$ (in <sup>2</sup> ):	3.07
Volume, $V$ (in <sup>3</sup> ):	13.73
Wet Mass of Specimen (lb):	1.3
Moisture Content (%):	2.3
Dry Mass of Specimen (lb):	1.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	162.3
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	158.7

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psi): 2246  
Strain (%): 1.5



**NOTES:** Slightly strong, dark gray, SHALE.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-007-0-11, ST-24, Depth: 88.5 - 90.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

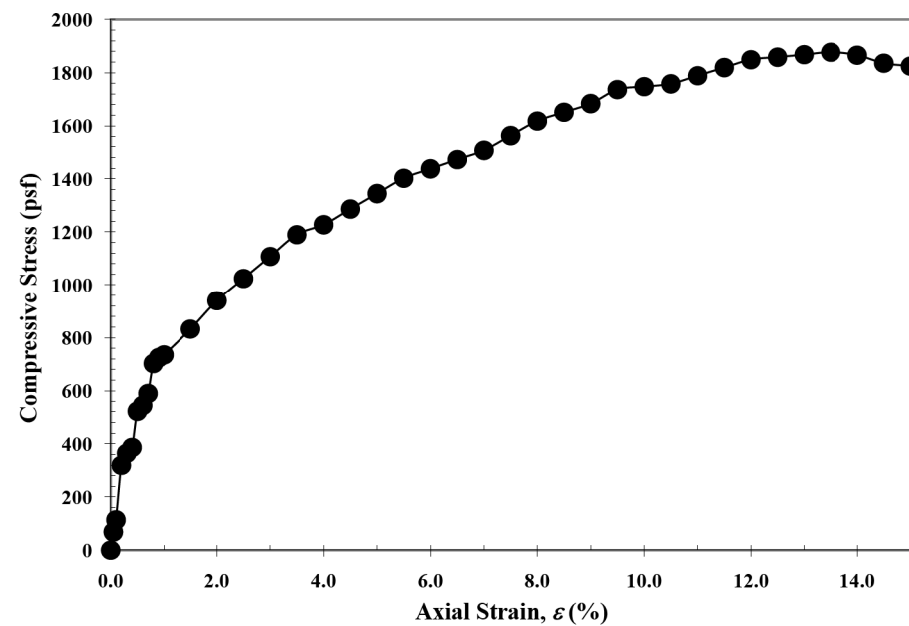
Average Dia., $D_{avg}$ (in):	2.83
Average Height, $H_{avg}$ (in):	5.96
Area, $A$ (in <sup>2</sup> ):	6.31
Volume, $V$ (in <sup>3</sup> ):	37.59
Wet Mass of Specimen (lb):	2.7
Moisture Content (%):	27.4
Dry Mass of Specimen (lb):	2.1
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	123.5
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	97.0

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): **1876.2**  
Strain (%): **13.5**



**Notes:** Medium Stiff, Gray, Silt, "And" Clay, Trace Sand, Wet.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-007-0-11, ST-26, Depth: 98.5 - 100.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

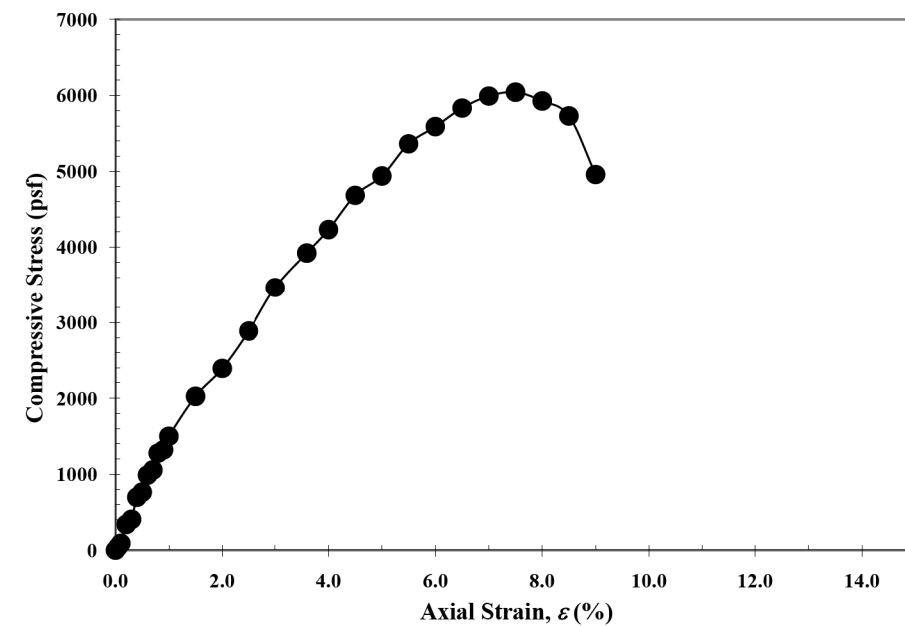
Average Dia., $D_{avg}$ (in):	2.85
Average Height, $H_{avg}$ (in):	5.96
Area, $A$ (in <sup>2</sup> ):	6.37
Volume, $V$ (in <sup>3</sup> ):	37.95
Wet Mass of Specimen (lb):	2.9
Moisture Content (%):	20.1
Dry Mass of Specimen (lb):	2.4
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	131.4
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	109.5

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): **6042.7**  
Strain (%): **7.5**



**Notes:** Very Stiff, Gray, Silt, Some Clay, Trace Fine Sand, Contains Many Silt Lenses, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-008-0-11, ST-17, Depth: 53.5 - 55.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

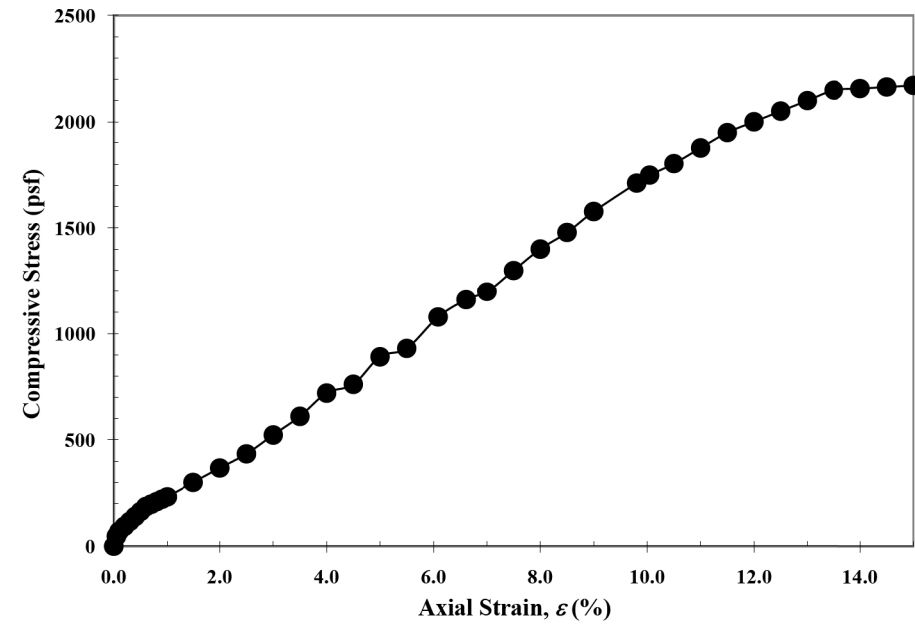
Average Dia., $D_{avg}$ (in):	2.80
Average Height, $H_{avg}$ (in):	5.73
Area, $A$ (in <sup>2</sup> ):	6.15
Volume, $V$ (in <sup>3</sup> ):	35.23
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	20.2
Dry Mass of Specimen (lb):	2.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	138.3
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	115.0

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 2170.8  
Strain (%): 15.0



**Notes:** Stiff, Brown, Silt, Some Clay, Trace Fine Sand, Wet.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-008-0-11, ST-26, Depth: 98.5 - 100.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

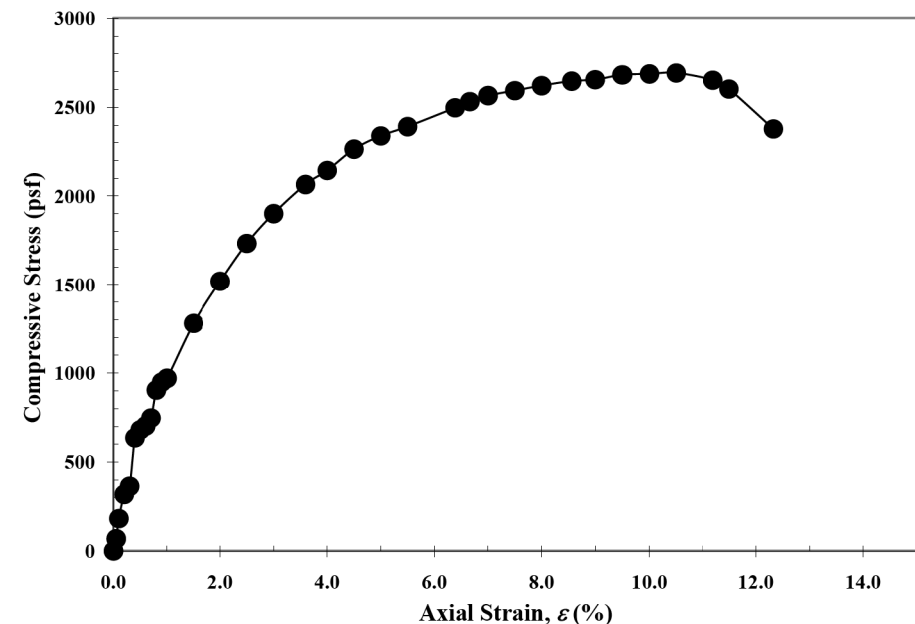
Average Dia., $D_{avg}$ (in):	2.84
Average Height, $H_{avg}$ (in):	5.84
Area, $A$ (in <sup>2</sup> ):	6.32
Volume, $V$ (in <sup>3</sup> ):	36.90
Wet Mass of Specimen (lb):	2.6
Moisture Content (%):	30.2
Dry Mass of Specimen (lb):	2.0
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	122.8
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	94.4

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 2692.9  
Strain (%): 10.5



**Notes:** Stiff, Gray, Silty Clay, Trace Sand, Trace Gravel, Moist.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-014-0-11, ST-19, Depth: 70.5 - 72.5 ft)  
Tested Date: 8/11/2011

**Specimen Properties**

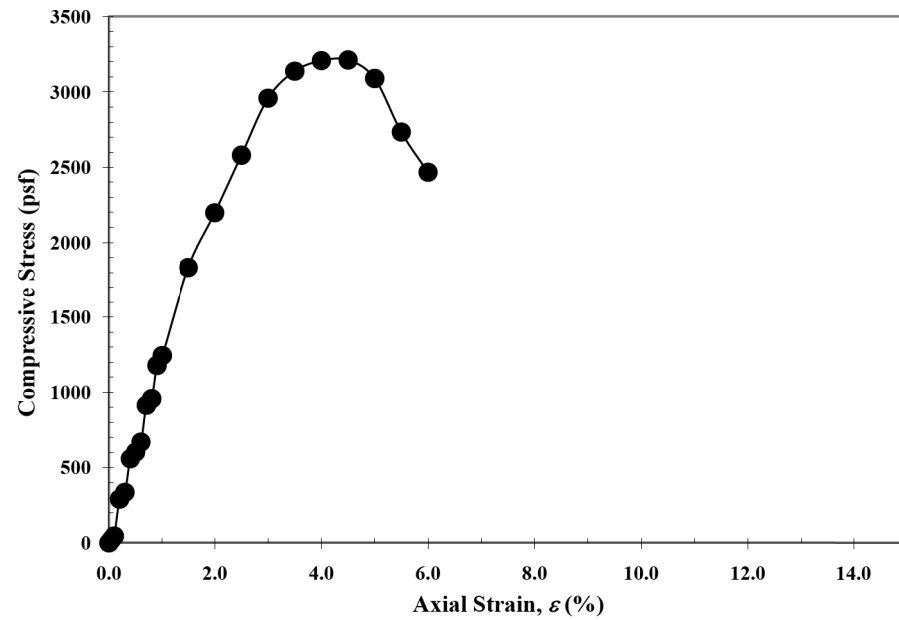
Average Dia., $D_{avg}$ (in):	2.86
Average Height, $H_{avg}$ (in):	6.00
Area, $A$ (in <sup>2</sup> ):	6.42
Volume, $V$ (in <sup>3</sup> ):	38.50
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	23.9
Dry Mass of Specimen (lb):	2.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	125.2
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	101.0

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 3212.6  
Strain (%): 4.5



**Notes:** Stiff, Gray, Silt, "And" Clay, Trace Sand, Contains Many Silt Lenses, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-014-0-11, ST-24, Depth: 108.5 - 110.5 ft)  
Tested Date: 8/11/2011

**Specimen Properties**

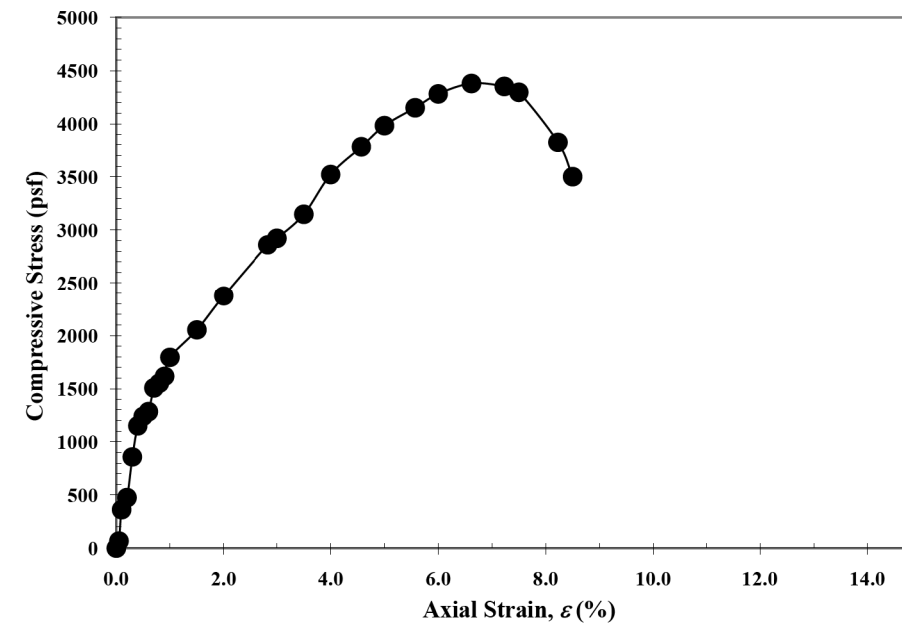
Average Dia., $D_{avg}$ (in):	2.84
Average Height, $H_{avg}$ (in):	5.91
Area, $A$ (in <sup>2</sup> ):	6.36
Volume, $V$ (in <sup>3</sup> ):	37.55
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	20.0
Dry Mass of Specimen (lb):	2.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	128.8
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	107.3

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 4379.0  
Strain (%): 6.6



**Notes:** Very Stiff, Gray, Silt and Clay, Trace Sand, Moist.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-015-0-11, ST-18, Depth: 61.5 - 63.5 ft)  
Tested Date: 8/10/2011

**Specimen Properties**

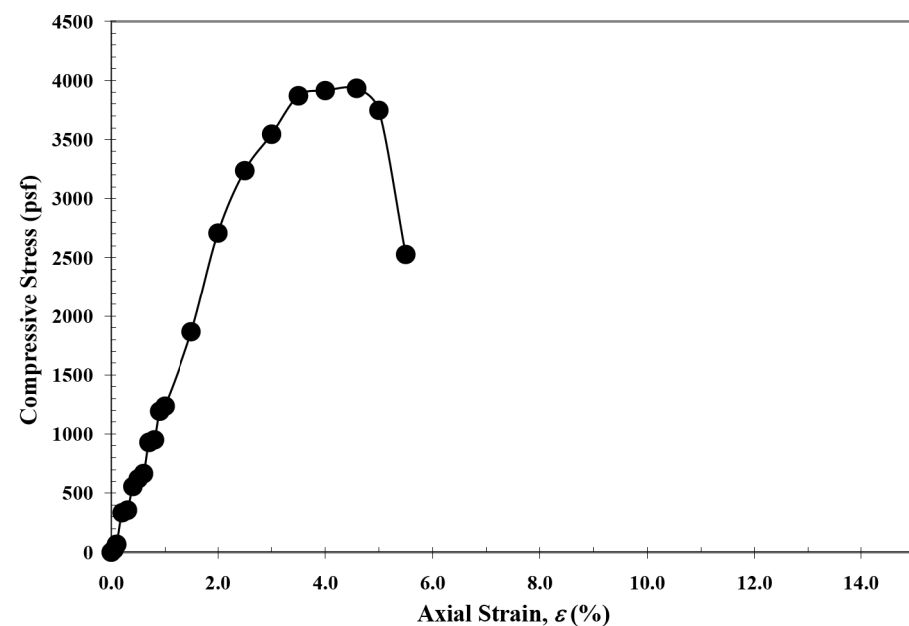
Average Dia., $D_{avg}$ (in):	2.87
Average Height, $H_{avg}$ (in):	5.98
Area, $A$ (in <sup>2</sup> ):	6.46
Volume, $V$ (in <sup>3</sup> ):	38.62
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	22.7
Dry Mass of Specimen (lb):	2.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	123.8
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	101.0

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 3933.6  
Strain (%): 4.6



**Notes:** Stiff, Gray, Silt and Clay, Trace Sand, Contains Many Silt Lenses, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-015-0-11, ST-22, Depth: 92.0 - 94.0 ft)  
Tested Date: 8/11/2011

**Specimen Properties**

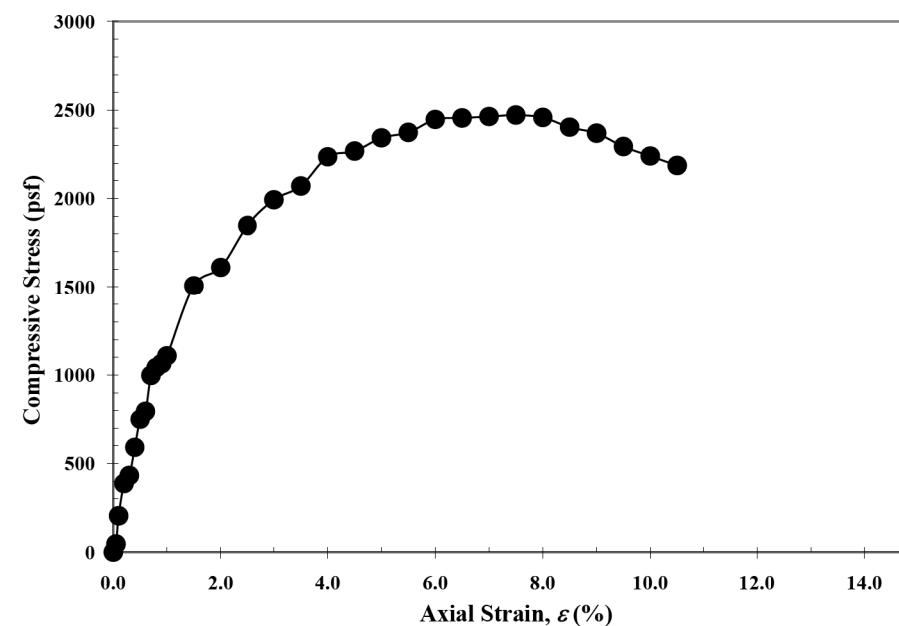
Average Dia., $D_{avg}$ (in):	2.83
Average Height, $H_{avg}$ (in):	6.07
Area, $A$ (in <sup>2</sup> ):	6.31
Volume, $V$ (in <sup>3</sup> ):	38.26
Wet Mass of Specimen (lb):	2.6
Moisture Content (%):	27.6
Dry Mass of Specimen (lb):	2.1
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	119.5
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	93.7

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 2471.7  
Strain (%): 7.5



**Notes:** Stiff, Gray, Silt and Clay, Trace Sand, Trace Gravel, Contains Many Silt Lenses, Moist.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-015-1-11, ST-22, Depth: 101.0 - 103.0 ft)  
Tested Date: 8/10/2011

**Specimen Properties**

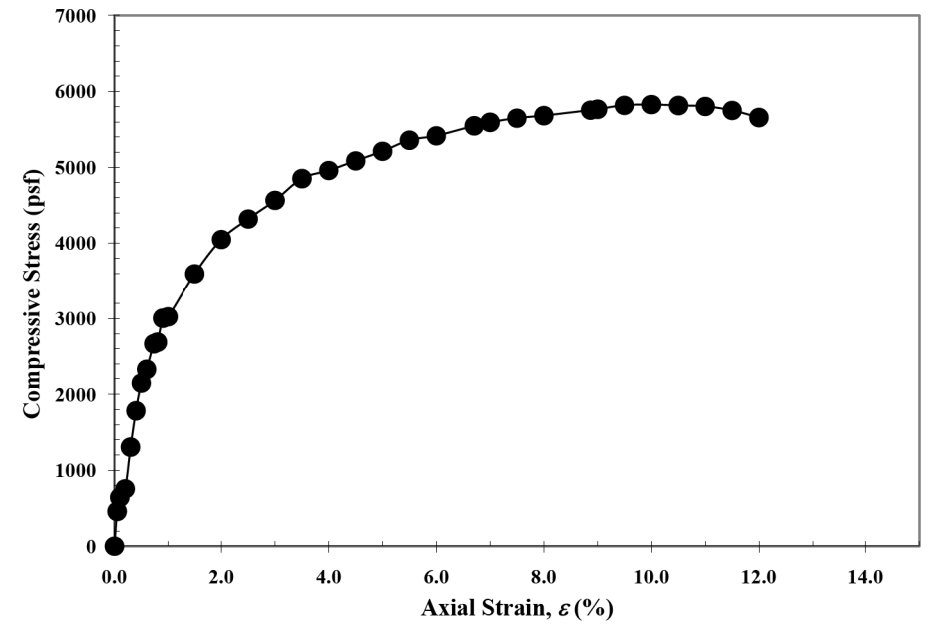
Average Dia., $D_{avg}$ (in):	2.83
Average Height, $H_{avg}$ (in):	6.01
Area, $A$ (in <sup>2</sup> ):	6.27
Volume, $V$ (in <sup>3</sup> ):	37.71
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	26.3
Dry Mass of Specimen (lb):	2.2
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	126.0
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	99.8

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): **5825.1**  
Strain (%): **10.0**



**Notes:** Very Stiff, Gray, Silt and Clay, Trace Sand, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-016-0-11, ST-18, Depth: 68.5 - 70.5 ft)  
Tested Date: 8/10/2011

**Specimen Properties**

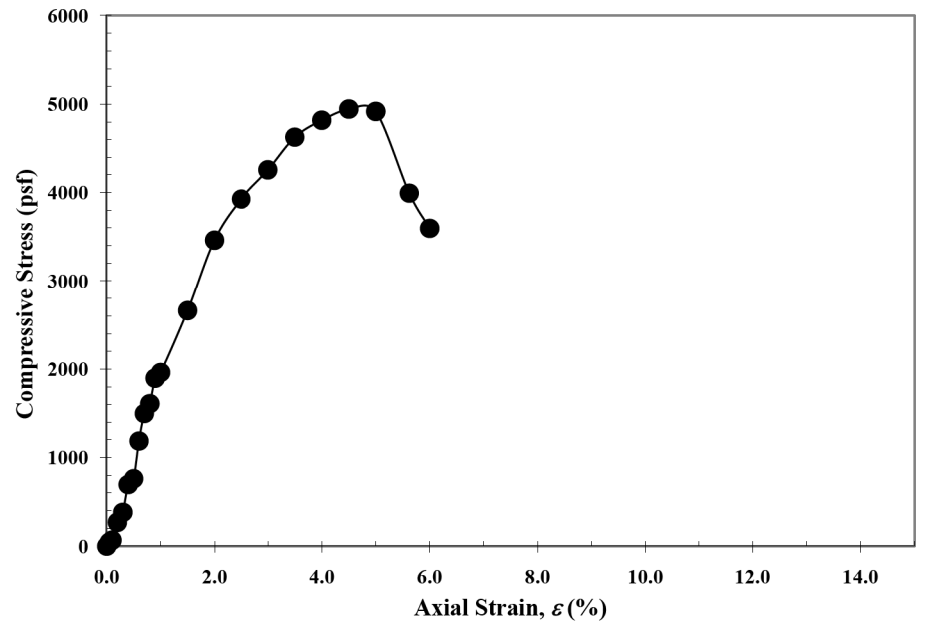
Average Dia., $D_{avg}$ (in):	2.85
Average Height, $H_{avg}$ (in):	5.96
Area, $A$ (in <sup>2</sup> ):	6.40
Volume, $V$ (in <sup>3</sup> ):	38.13
Wet Mass of Specimen (lb):	2.7
Moisture Content (%):	20.2
Dry Mass of Specimen (lb):	2.3
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	124.3
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	103.4

**Final Specimen Figure**

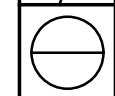


**Results**

Unconfined Compressive Strength (psf): **4943.3**  
Strain (%): **4.5**



**Notes:** Very Stiff, Gray, Silt, Some Clay, Contains Many Silt Lenses, Wet.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-016-0-11, ST-22, Depth: 101.5 - 103.5 ft)  
Tested Date: 8/12/2011

**Specimen Properties**

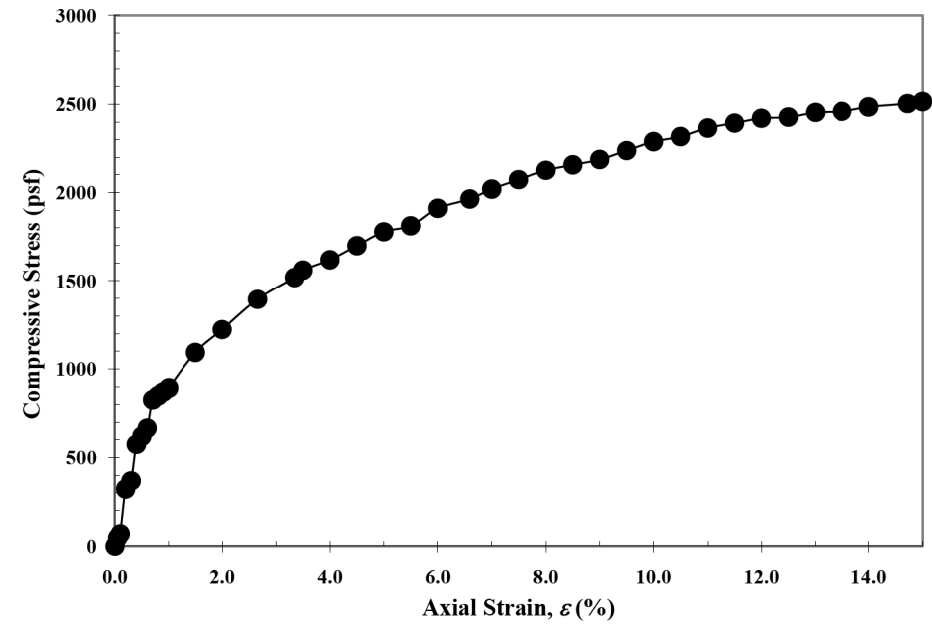
Average Dia., $D_{avg}$ (in):	2.82
Average Height, $H_{avg}$ (in):	5.98
Area, $A$ (in <sup>2</sup> ):	6.23
Volume, $V$ (in <sup>3</sup> ):	37.25
Wet Mass of Specimen (lb):	2.6
Moisture Content (%):	30.6
Dry Mass of Specimen (lb):	2.0
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	121.5
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	93.0

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 2514.5  
Strain (%): 15.0



**Notes:** Stiff, Gray, Silty Clay, Trace Fine Sand, Contains Few Silt Lenses, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-024-0-11, ST-19, Depth: 70.5 - 72.5 ft)  
Tested Date: 8/10/2011

**Specimen Properties**

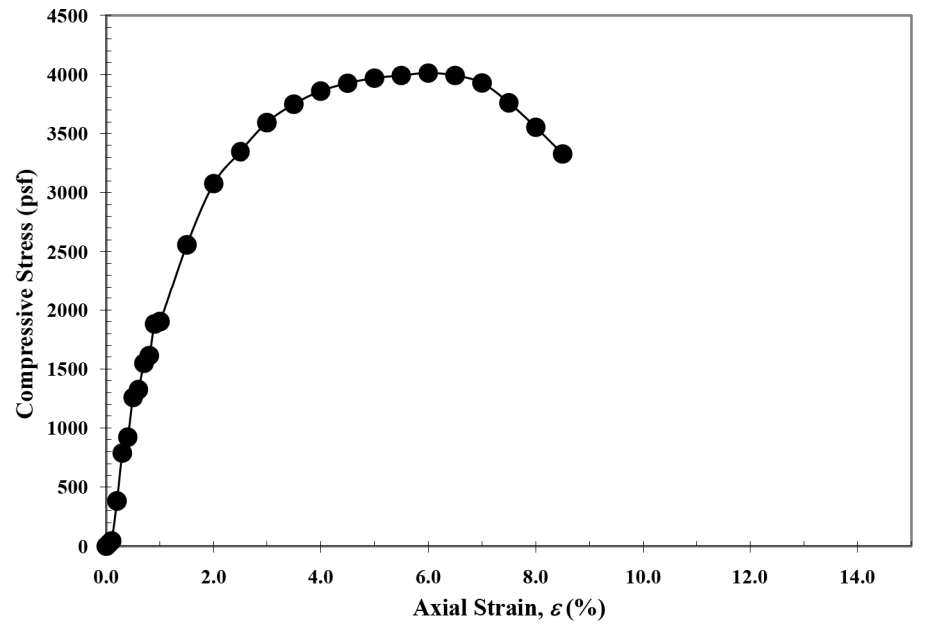
Average Dia., $D_{avg}$ (in):	2.85
Average Height, $H_{avg}$ (in):	5.93
Area, $A$ (in <sup>2</sup> ):	6.38
Volume, $V$ (in <sup>3</sup> ):	37.78
Wet Mass of Specimen (lb):	2.6
Moisture Content (%):	29.5
Dry Mass of Specimen (lb):	2.0
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	120.6
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	93.2

**Final Specimen Figure**

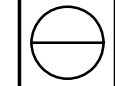


**Results**

Unconfined Compressive Strength (psf): 4012.4  
Strain (%): 6.0



**Notes:** Very Stiff, Gray, Silty Clay, Trace Sand, Trace Gravel, Moist.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-025-0-11, ST-22, Depth: 90.5 - 92.5 ft)  
Tested Date: 8/11/2011

**Specimen Properties**

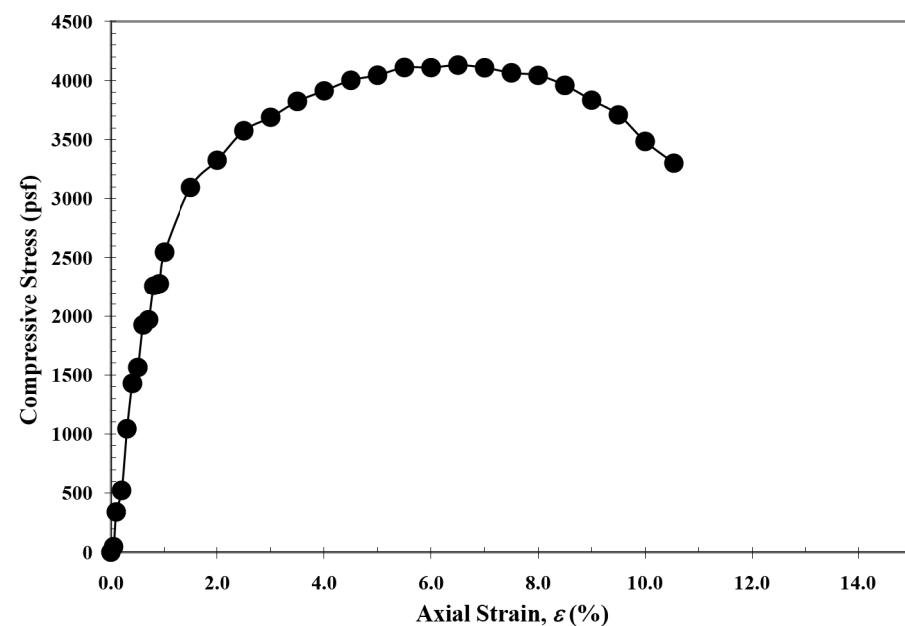
Average Dia., $D_{avg}$ (in):	2.84
Average Height, $H_{avg}$ (in):	5.94
Area, $A$ (in <sup>2</sup> ):	6.32
Volume, $V$ (in <sup>3</sup> ):	37.57
Wet Mass of Specimen (lb):	2.7
Moisture Content (%):	28.2
Dry Mass of Specimen (lb):	2.1
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	122.6
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	95.6

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 4130.2  
Strain (%): 6.5



**Notes:** Very Stiff, Gray, Silty Clay, Trace Sand, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-026-0-11, ST-20, Depth: 70.5 - 72.5 ft)  
Tested Date: 8/10/2011

**Specimen Properties**

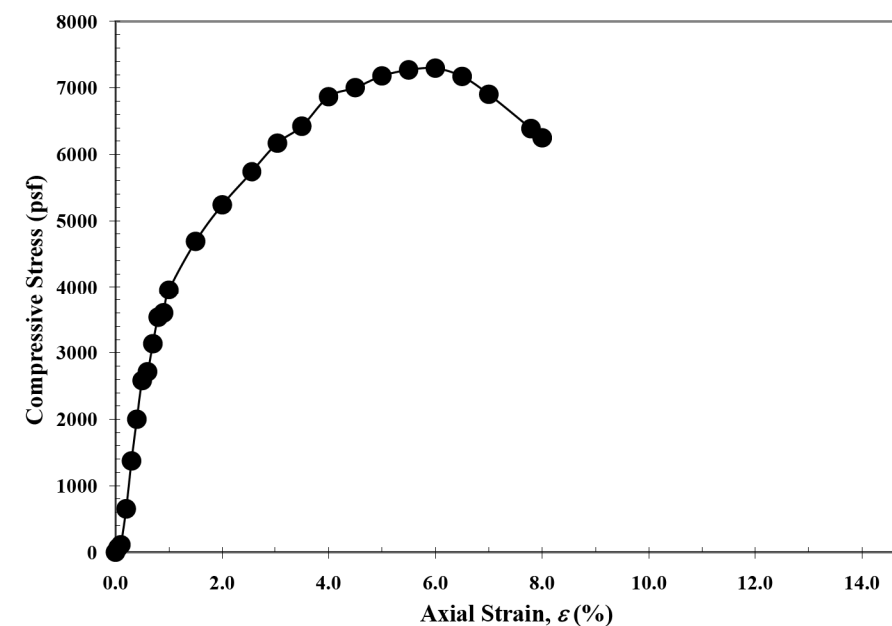
Average Dia., $D_{avg}$ (in):	2.85
Average Height, $H_{avg}$ (in):	5.98
Area, $A$ (in <sup>2</sup> ):	6.38
Volume, $V$ (in <sup>3</sup> ):	38.17
Wet Mass of Specimen (lb):	2.9
Moisture Content (%):	20.8
Dry Mass of Specimen (lb):	2.4
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	131.3
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	108.7

**Final Specimen Figure**

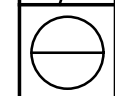


**Results**

Unconfined Compressive Strength (psf): 7295.9  
Strain (%): 6.0



**Notes:** Very Stiff, Gray, Silt and Clay, Trace Sand, Wet.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-026-0-11, ST-25, Depth: 110.5 - 112.5 ft)  
Tested Date: 8/11/2011

**Specimen Properties**

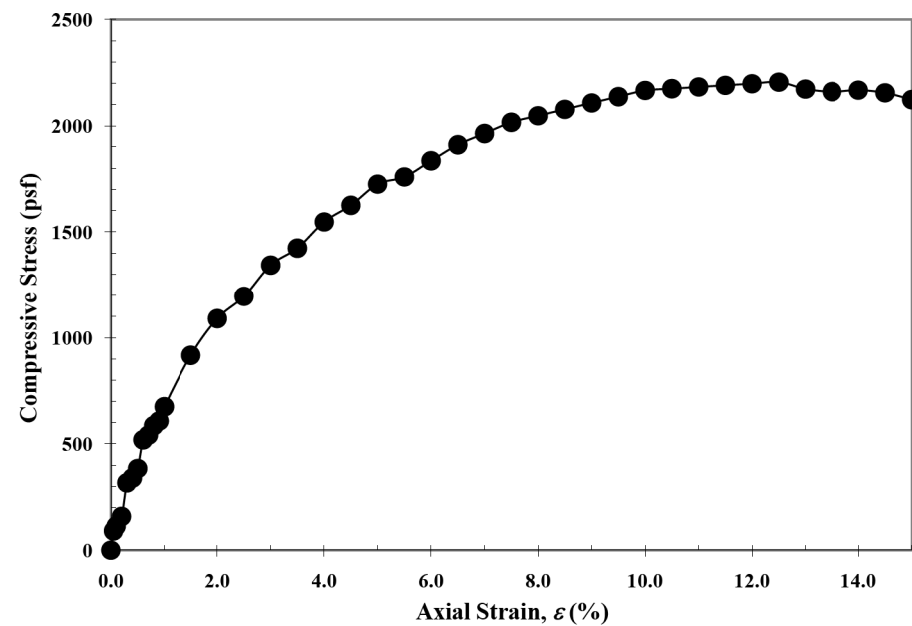
Average Dia., $D_{avg}$ (in):	2.84
Average Height, $H_{avg}$ (in):	5.59
Area, $A$ (in <sup>2</sup> ):	6.34
Volume, $V$ (in <sup>3</sup> ):	35.43
Wet Mass of Specimen (lb):	2.5
Moisture Content (%):	32.0
Dry Mass of Specimen (lb):	1.9
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	120.0
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	90.9

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 2204.9  
Strain (%): 12.5



**Notes:** Stiff, Gray, Clay, Little Silt, Trace Sand, Moist.

**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-031-0-11, ST-22, Depth: 78.5 - 80.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

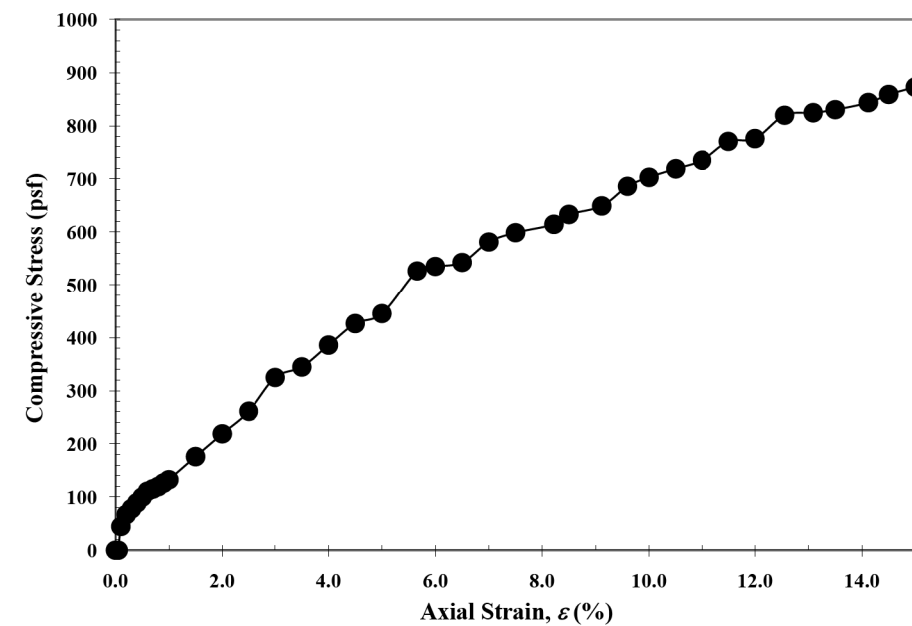
Average Dia., $D_{avg}$ (in):	2.87
Average Height, $H_{avg}$ (in):	5.81
Area, $A$ (in <sup>2</sup> ):	6.45
Volume, $V$ (in <sup>3</sup> ):	37.51
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	25.0
Dry Mass of Specimen (lb):	2.2
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	127.2
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	101.8

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): 872.8  
Strain (%): 15.0



**Notes:** Soft, Brown, Silt and Clay, Trace Sand, Wet. Fuel Odor.



**Unconfined Compressive Strength Test**

(Project: CUY-90-14.90, Boring Location: B-039-0-11, ST-26, Depth: 98.5 - 100.5 ft)  
Tested Date: 8/5/2011

**Specimen Properties**

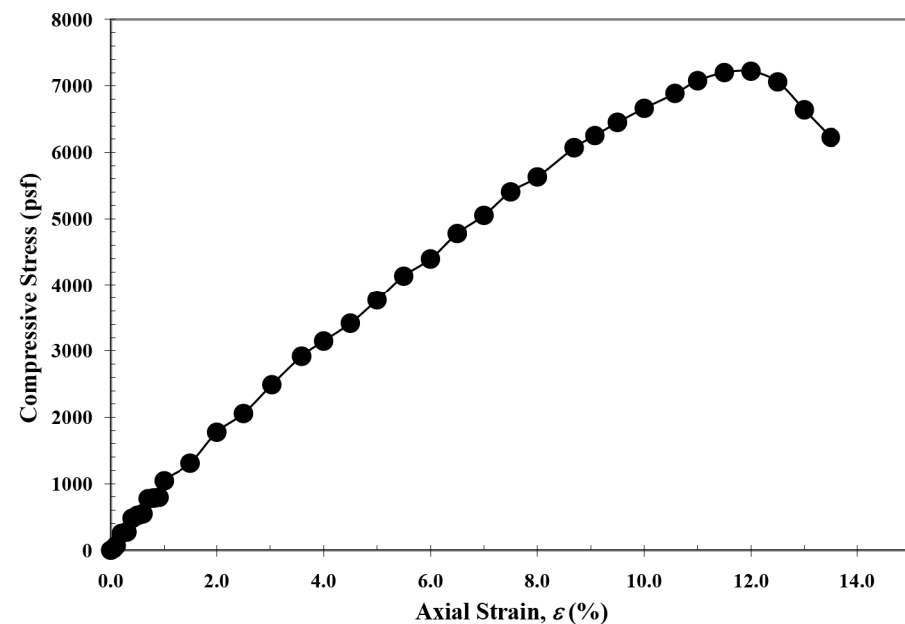
Average Dia., $D_{avg}$ (in):	2.83
Average Height, $H_{avg}$ (in):	5.94
Area, $A$ (in <sup>2</sup> ):	6.28
Volume, $V$ (in <sup>3</sup> ):	37.31
Wet Mass of Specimen (lb):	2.8
Moisture Content (%):	19.8
Dry Mass of Specimen (lb):	2.4
Wet Unit Weight, $\gamma$ (lb/ft <sup>3</sup> ):	131.0
Dry Unit Weight, $\gamma_d$ (lb/ft <sup>3</sup> ):	109.3

**Final Specimen Figure**



**Results**

Unconfined Compressive Strength (psf): **7220.5**  
Strain (%): **12.0**

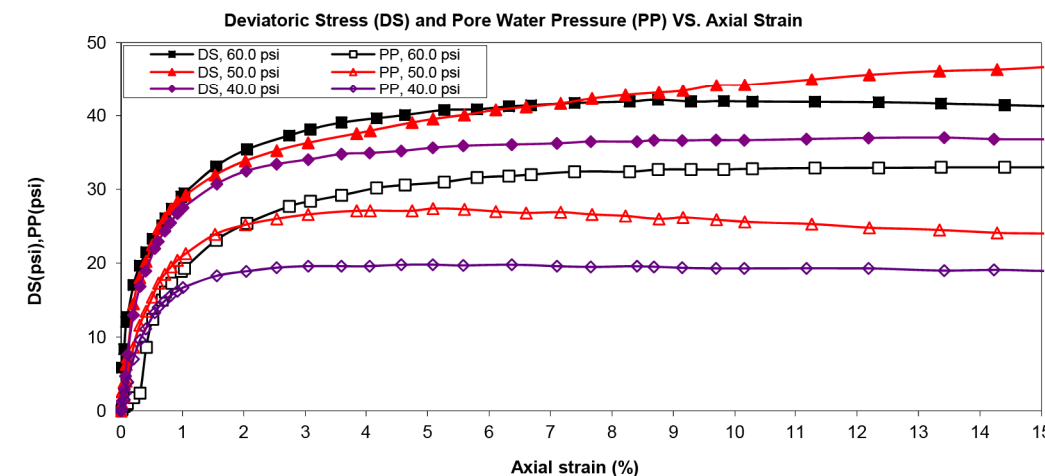
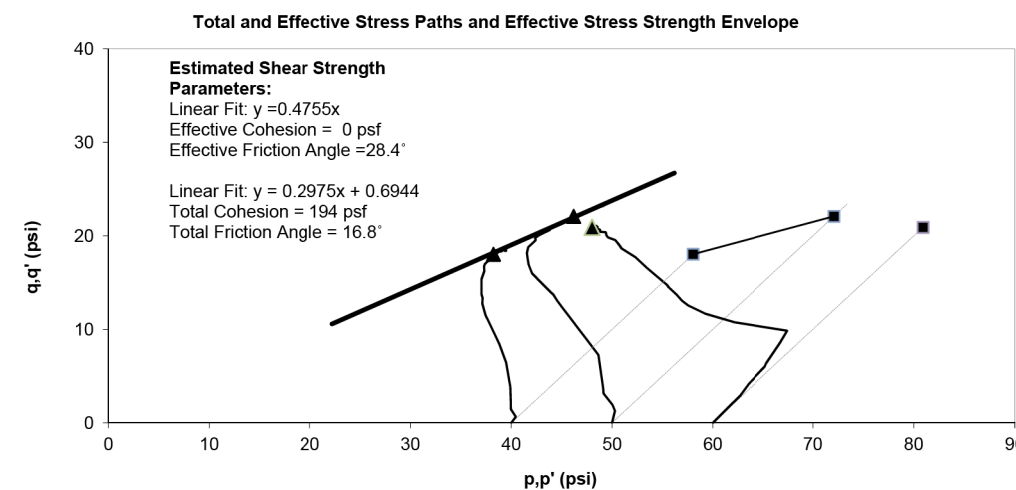


**Notes:** Very Stiff, Brown, Silt, Some Clay, Moist.

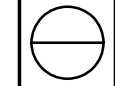
Project: CUY-90-14.90 Project ID: 82119  
Sample ID: Boring B-013-0-11, ST-20 (81.0'-83.0')  
Description: Medium Stiff, Gray, Silt and Clay, Contains Few Silt Lenses, Moist.

Sample No.	Height (in)	Diameter (in)	Moisture (%)	Bulk Density (pcf)	Dry Density (pcf)
1	5.53	2.84	25.2%	134.2	107.2
2	5.97	2.86	24.7%	125.6	100.8
3	5.97	2.84	26.0%	122.3	97.0

Failure Criterion : Maximum Effective Principal Stress Ratio



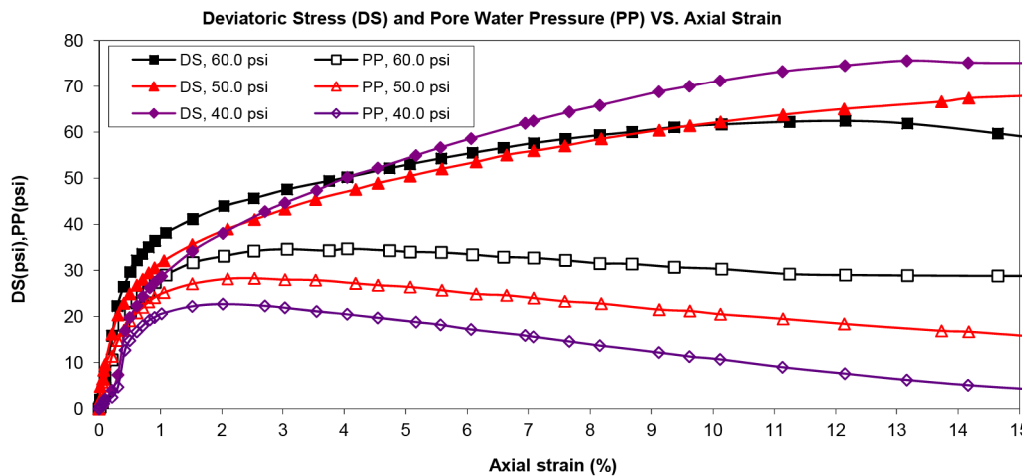
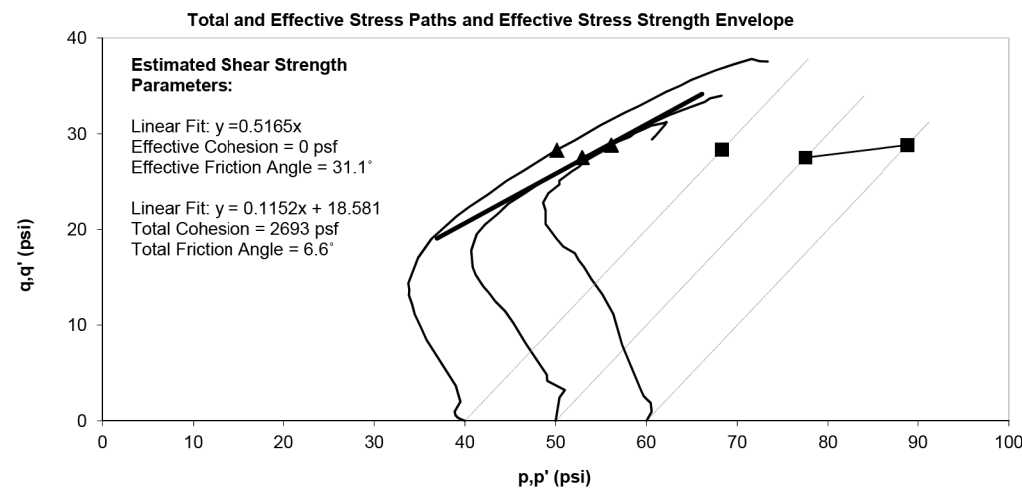
Assumed Specific Gravity  $G_s = 2.70$ .



Project: CUY-90-14.90 Project ID: 82119  
Sample ID: Boring B-013-1-11, ST-18 (61.5'-63.5')  
Description: Soft to very stiff, gray, Silt, "and" Clay, contains many clay lenses, wet.

Sample No.	Height (in)	Diameter (in)	Moisture (%)	Bulk Density (pcf)	Dry Density (pcf)
1	5.27	2.85	22.1%	127.1	104.1
2	5.27	2.85	24.8%	129.1	103.4
3	5.78	2.84	22.5%	128.5	104.9

Failure Criterion : Maximum Effective Principal Stress Ratio

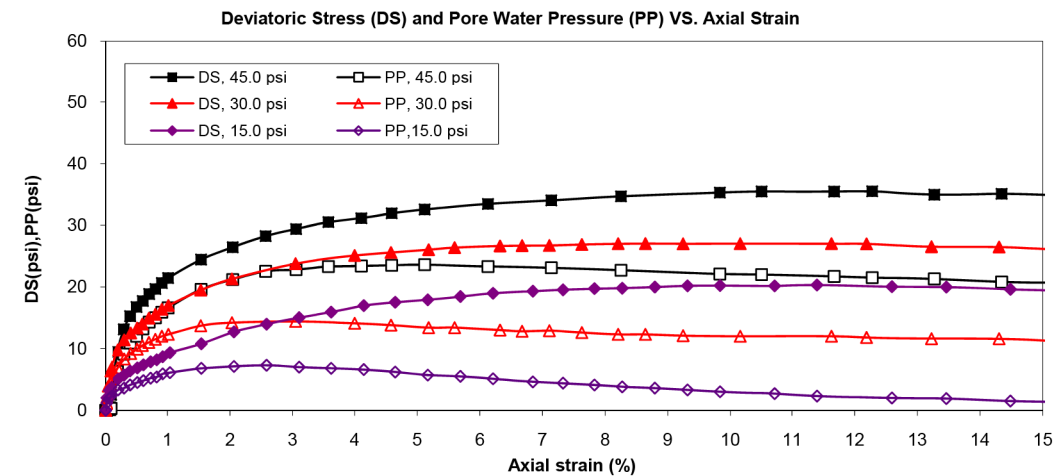
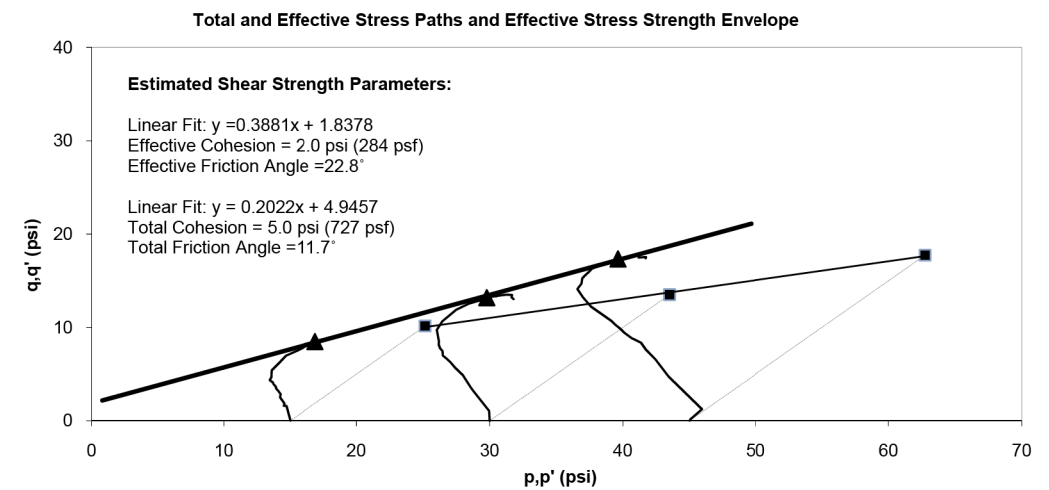


Assumed Specific Gravity  $G_s = 2.70$ .

Project: CUY-90-14.90 Project ID: 82119  
Sample ID: Boring B-017-0-11, ST-13 (33.0'-35.0')  
Description: Medium stiff, gray, Silty Clay, trace sand, trace gravel, moist.

Sample No.	Height (in)	Diameter (in)	Moisture (%)	Bulk Density (pcf)	Dry Density (pcf)
1	5.37	2.85	22.0	128.0	105.0
2	4.90	2.84	21.1	131.8	108.9
3	5.05	2.82	22.9	127.9	104.1

Failure Criterion : Maximum Effective Principal Stress Ratio

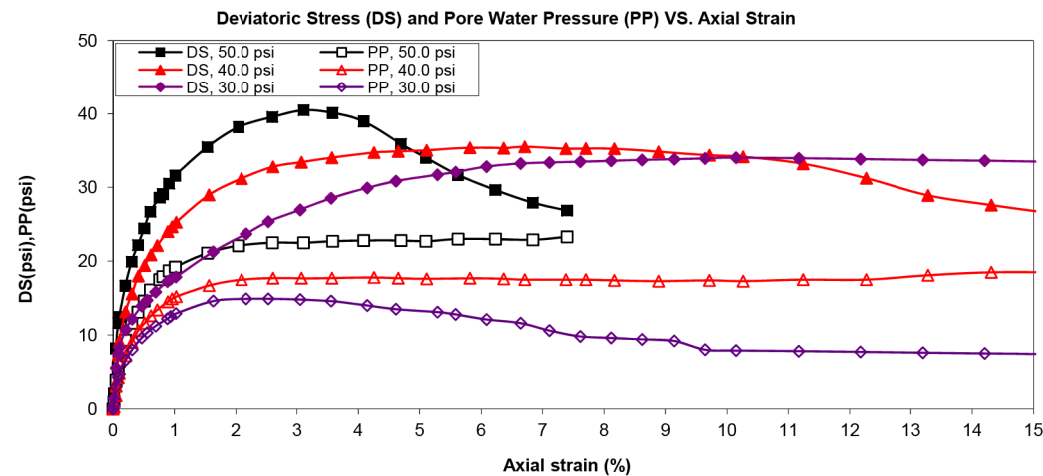
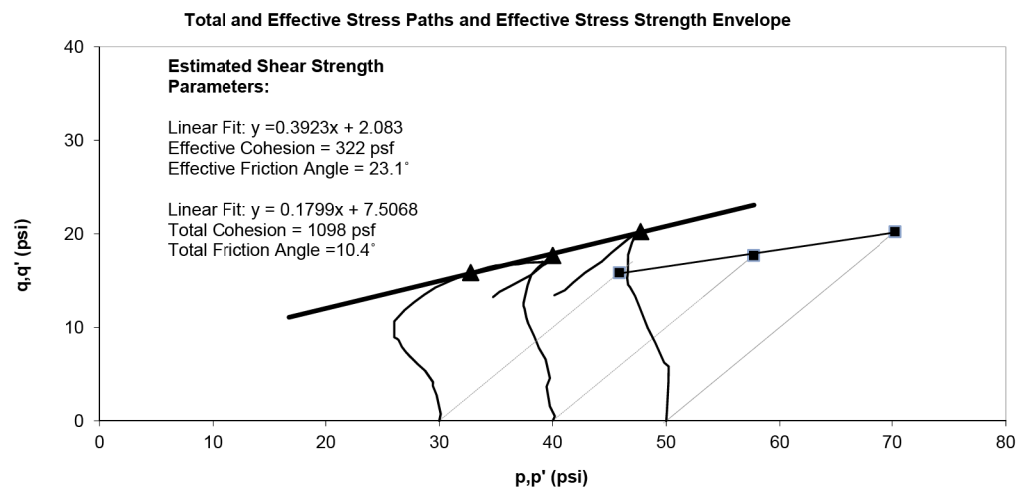


Assumed Specific Gravity  $G_s = 2.70$ . Height to diameter ratio is smaller than ASTM requirement (~1.8:1).

Project: CUY-90-14.90 Project ID: 82119  
Sample ID: Boring B-028-0-11, ST-28 (103.5' - 105.5')  
Description: Medium Stiff, Gray, Silt and Clay, Trace Sand, Contains Few Silt Lenses, Moist.

Sample No.	Height (in)	Diameter (in)	Moisture (%)	Bulk Density (pcf)	Dry Density (pcf)
1	5.85	2.84	24.5%	125.9	101.1
2	5.96	2.86	25.7%	124.9	99.4
3	6.05	2.85	24.9%	127.5	102.1

Failure Criterion : Maximum Effective Principal Stress Ratio



Assumed Specific Gravity  $G_s = 2.70$ .

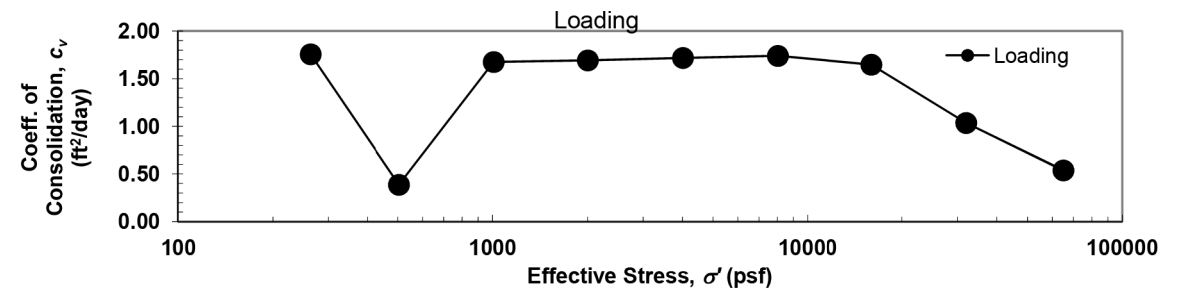
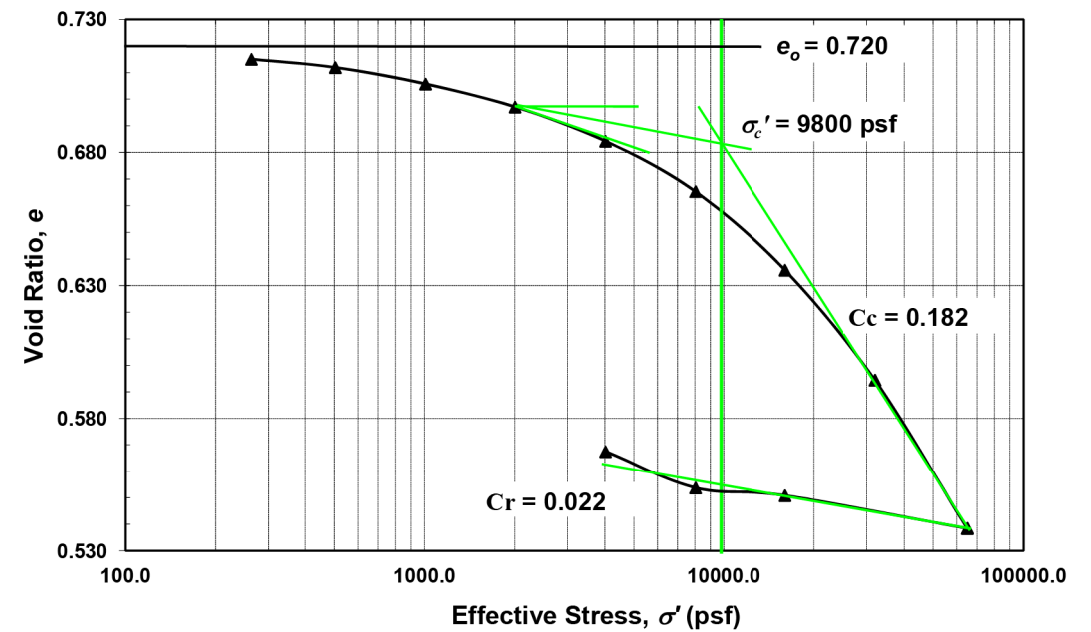
### Consolidation Test

Project Name: CUY-90-14.90 Prepared by: CH  
Source: B-013-0-11 (ST-20, 80.5 to 82.5 ft) Checked by: EC  
Description: Soft to medium stiff, Silt and Clay, contains few silt lenses. Date: 11/7/2011

Test Specification: ASTM D 2435-04  
Initial Void Ratio: 0.720 Initial Bulk Unit Weight (lb/ft<sup>3</sup>): 123.0  
In-situ Vertical Effective Stress: 7000 psf Dry Unit Weight (lb/ft<sup>3</sup>): 98.0

Compression and Swelling Index  
Compression Index ( $C_c$ ): 0.182 Preconsolidation Pressure ( $\sigma_c'$ ): 9800 psf  
Recompression Index ( $C_r$ ): 0.022 Over-Consolidation Ratio (OCR): 1.40

#### Consolidation Curve



Barr Prevost  
2800 Corporate Exchange Dr.  
Suite 240, Columbus, OH 43231

Contact Information  
Voice: 614-714-0270  
Fax: 614-714-0323

### Consolidation Test

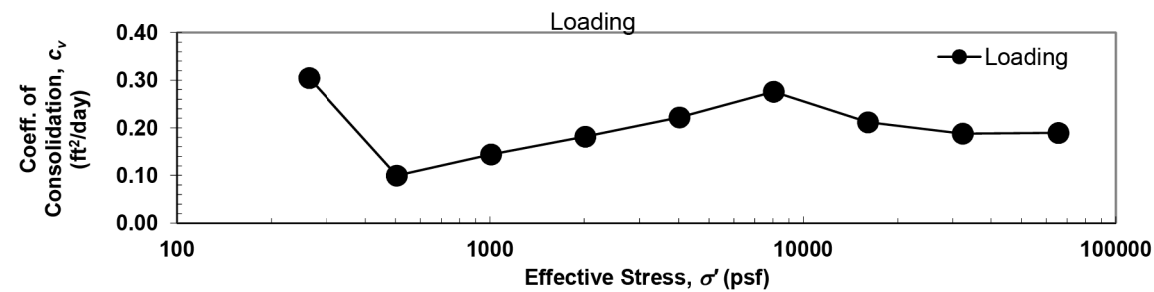
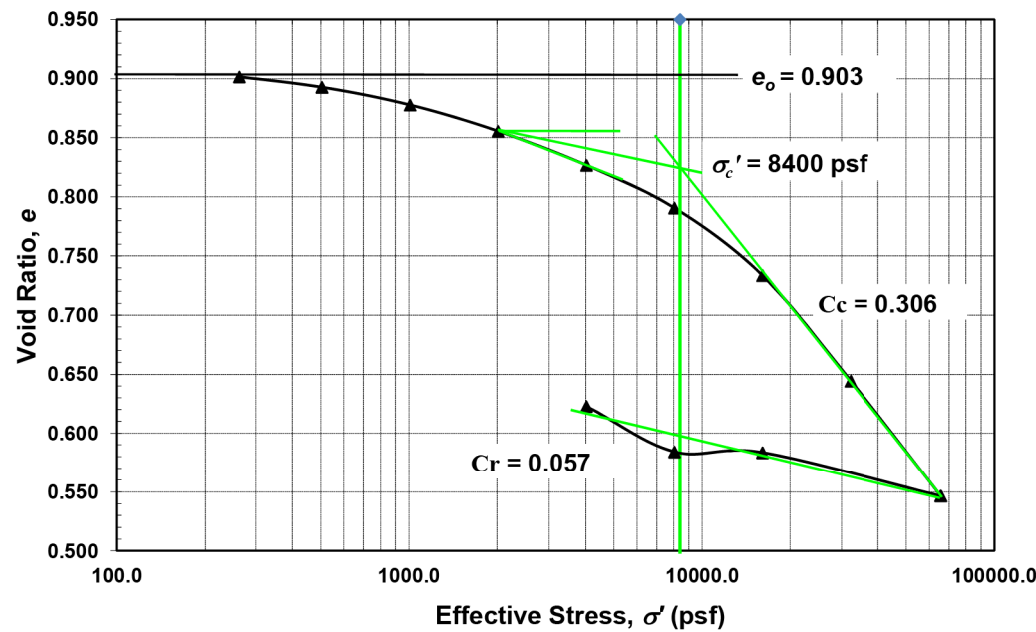
Project Name: CUY-90-14.90 Prepared by: CH  
Source: B-013-1-11 (ST-22, 90.5 to 92.5 ft) Checked by: EC  
Description: Very soft to soft, Silty Clay, contains many silt lenses. Date: 11/2/2011

Test Specification: ASTM D 2435-04  
Initial Void Ratio: 0.903 Initial Bulk Unit Weight (lb/ft<sup>3</sup>): 118.7  
In-situ Vertical Effective Stress: 6100 psf Dry Unit Weight (lb/ft<sup>3</sup>): 88.6

#### Compression and Swelling Index

Compression Index ( $C_c$ ): 0.306 Preconsolidation Pressure ( $\sigma_c'$ ): 8400 psf  
Recompression Index ( $C_r$ ): 0.057 Over-Consolidation Ratio (OCR): 1.38

#### Consolidation Curve



Barr Prevost  
2800 Corporate Exchange Dr.  
Suite 240, Columbus, OH 43231

Contact Information  
Voice: 614-714-0270  
Fax: 614-714-0323

### Consolidation Test

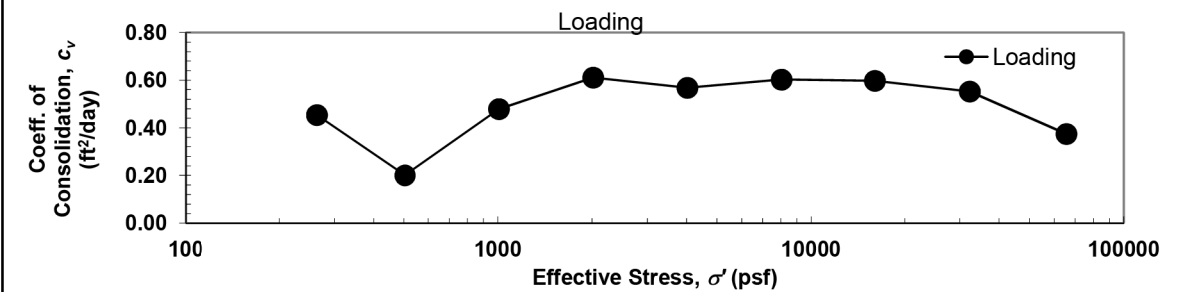
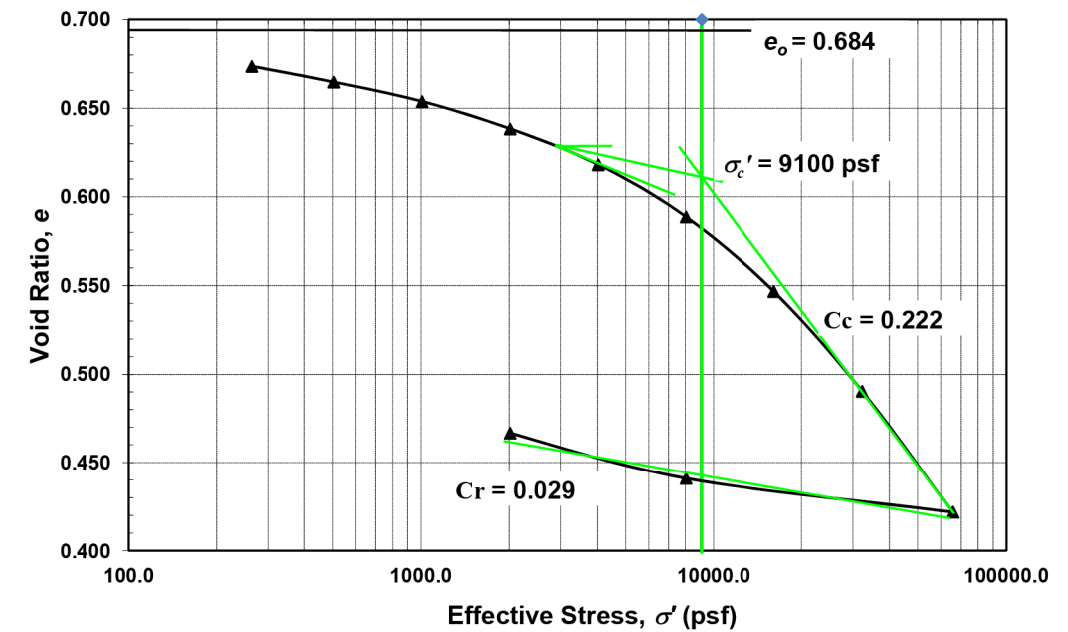
Project Name: CUY-90-14.90 Prepared by: CH  
Source: B-016-0-11 (ST-22, 101.6 to 101.7 ft) Checked by: EC  
Description: Very soft to soft, Silty Clay, trace fine sand. Date: 10/21/2011

Test Specification: ASTM D 2435-04  
Initial Void Ratio: 0.684 Initial Bulk Unit Weight (lb/ft<sup>3</sup>): 120.1  
In-situ Vertical Effective Stress: 7100 psf Dry Unit Weight (lb/ft<sup>3</sup>): 100.1

#### Compression and Swelling Index

Compression Index ( $C_c$ ): 0.222 Preconsolidation Pressure ( $\sigma_c'$ ): 9100 psf  
Recompression Index ( $C_r$ ): 0.029 Over-Consolidation Ratio (OCR): 1.28

#### Consolidation Curve



Barr Prevost  
2800 Corporate Exchange Dr.  
Suite 240, Columbus, OH 43231

Contact Information  
Voice: 614-714-0270  
Fax: 614-714-0323

### Consolidation Test

Project Name: CUY-90-14.90  
Source: B-028-0-11 (ST-23, 78.7 to 78.8 ft)  
Description: Medium stiff to hard, Silt and Clay, trace sand.

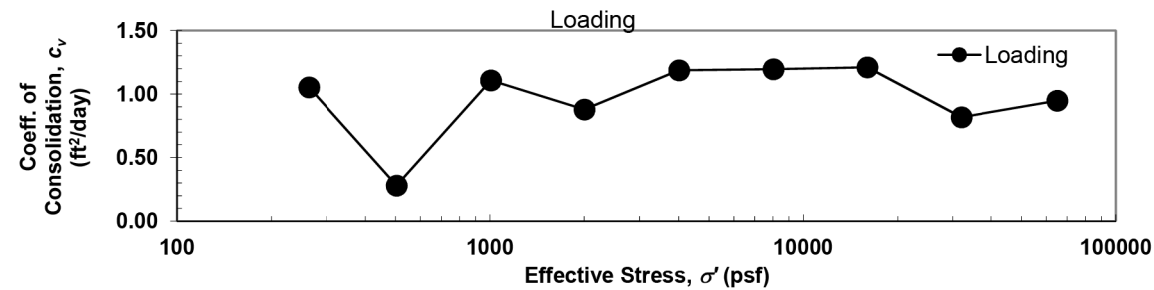
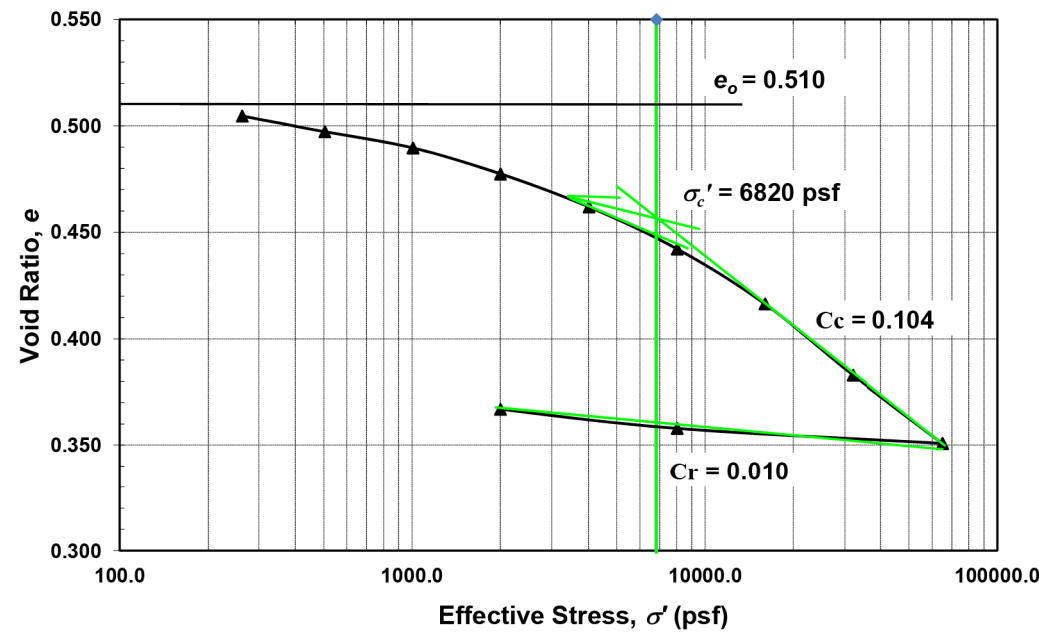
Prepared by: CH  
Checked by: EC  
Date: 10/21/2011

Test Specification: ASTM D 2435-04  
Initial Void Ratio: 0.510 Initial Bulk Unit Weight (lb/ft<sup>3</sup>): 126.3  
In-situ Vertical Effective Stress: 6820 psf Dry Unit Weight (lb/ft<sup>3</sup>): 111.6

#### Compression and Swelling Index

Compression Index ( $C_c$ ): 0.104 Preconsolidation Pressure ( $\sigma_c'$ ): 6820 psf  
Recompression Index ( $C_r$ ): 0.010 Over-Consolidation Ratio (OCR): 1.00

#### Consolidation Curve



Barr Prevost  
2800 Corporate Exchange Dr.  
Suite 240, Columbus, OH 43231

Contact Information  
Voice: 614-714-0270  
Fax: 614-714-0323

### Consolidation Test

Project Name: CUY-90-14.90  
Source: B-029-0-11 (ST-25, 88.6 to 90.6 ft)  
Description: Soft to hard, Silt and Clay, contains silt lenses.

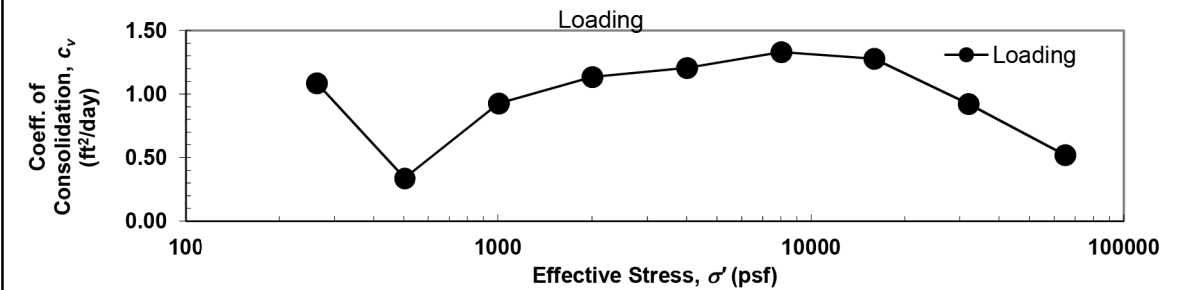
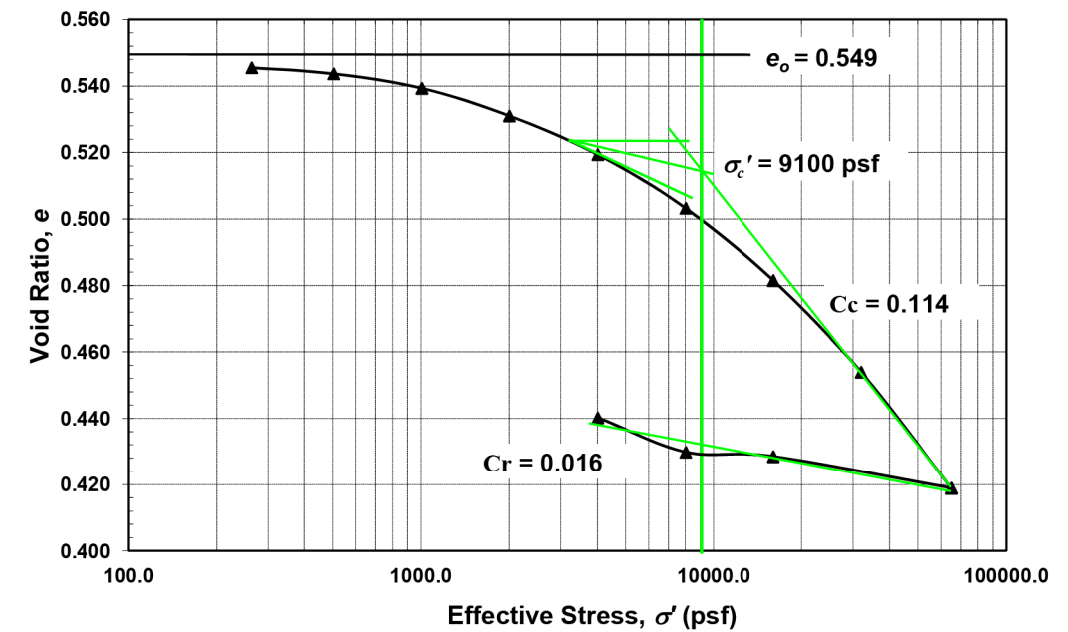
Prepared by: CH  
Checked by: EC  
Date: 11/2/2011

Test Specification: ASTM D 2435-04  
Initial Void Ratio: 0.549 Initial Bulk Unit Weight (lb/ft<sup>3</sup>): 130.8  
In-situ Vertical Effective Stress: 7700 psf Dry Unit Weight (lb/ft<sup>3</sup>): 108.8

#### Compression and Swelling Index

Compression Index ( $C_c$ ): 0.114 Preconsolidation Pressure ( $\sigma_c'$ ): 9100 psf  
Recompression Index ( $C_r$ ): 0.016 Over-Consolidation Ratio (OCR): 1.18

#### Consolidation Curve



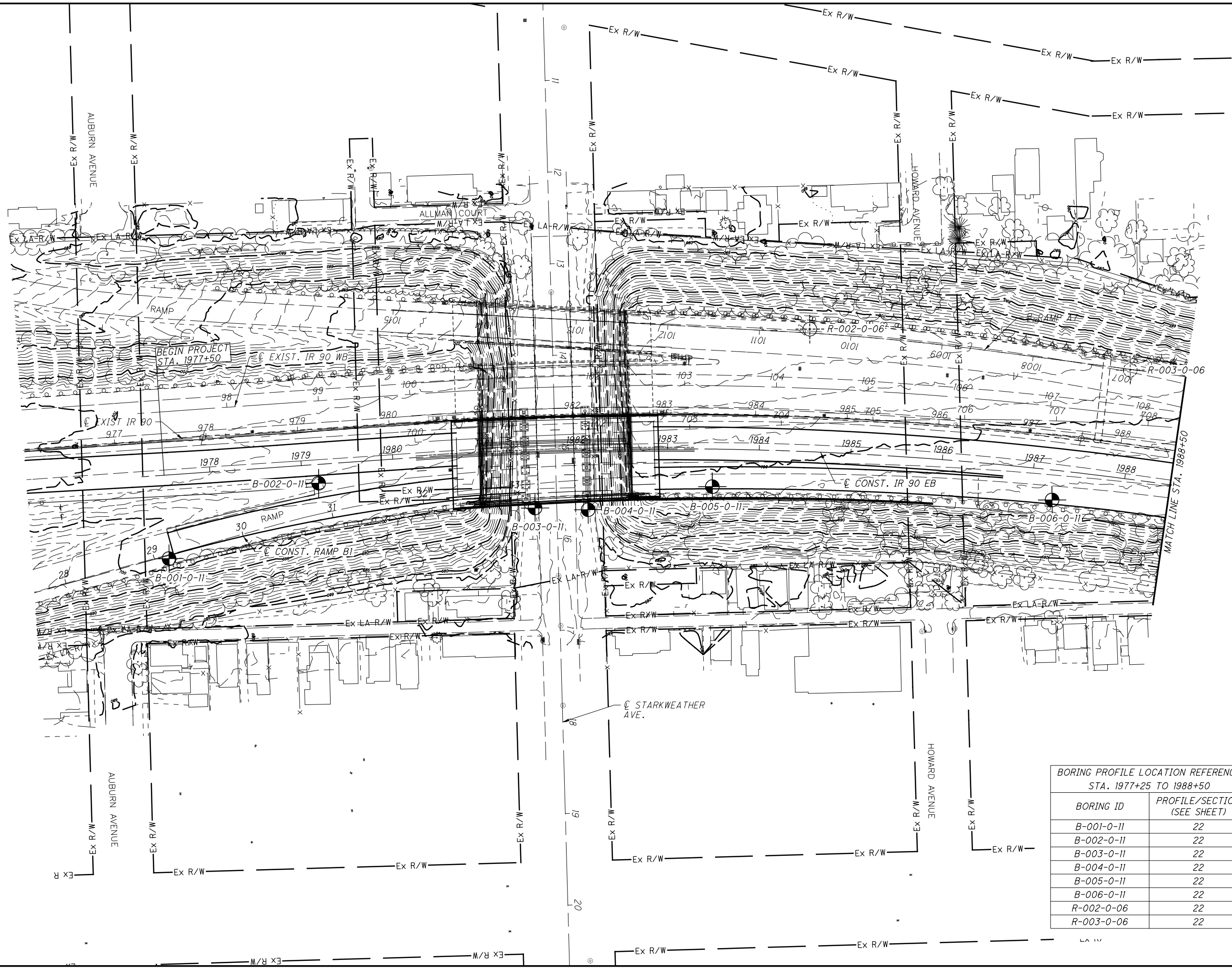
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CMH  
CHECKED  
JEP

SOIL PROFILE  
LABORATORY TEST DATA

CUY-90-14.90





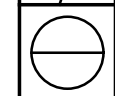
BORING PROFILE LOCATION REFERENCE  
STA. 1977+25 TO 1988+50

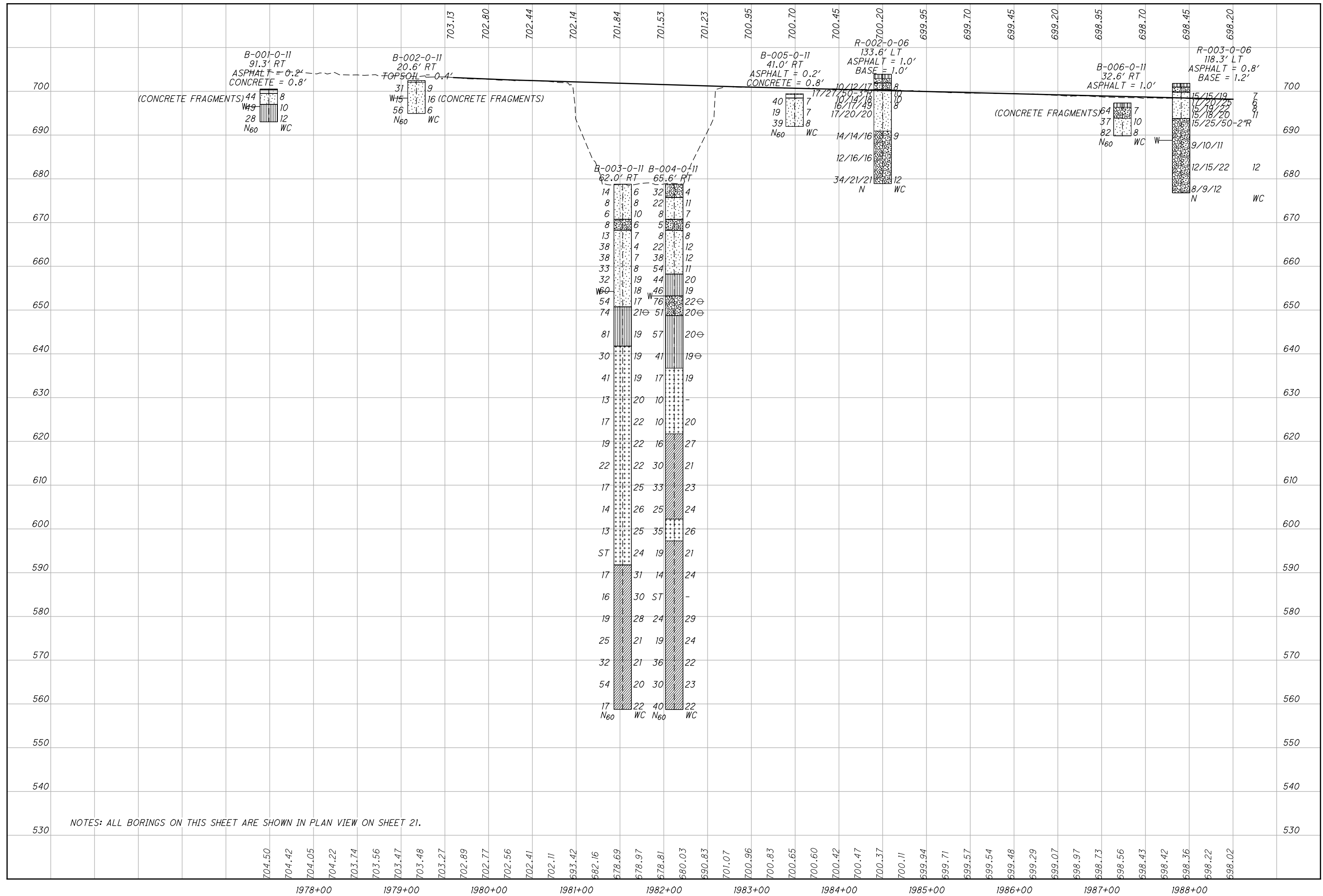
BORING ID	PROFILE/SECTION (SEE SHEET)
B-001-0-11	22
B-002-0-11	22
B-003-0-11	22
B-004-0-11	22
B-005-0-11	22
B-006-0-11	22
R-002-0-06	22
R-003-0-06	22



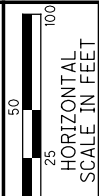
SOIL PLAN  
STA. 1977+25 TO STA. 1988+50 IR 90 EB

CUY-90-14.90





NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 21.



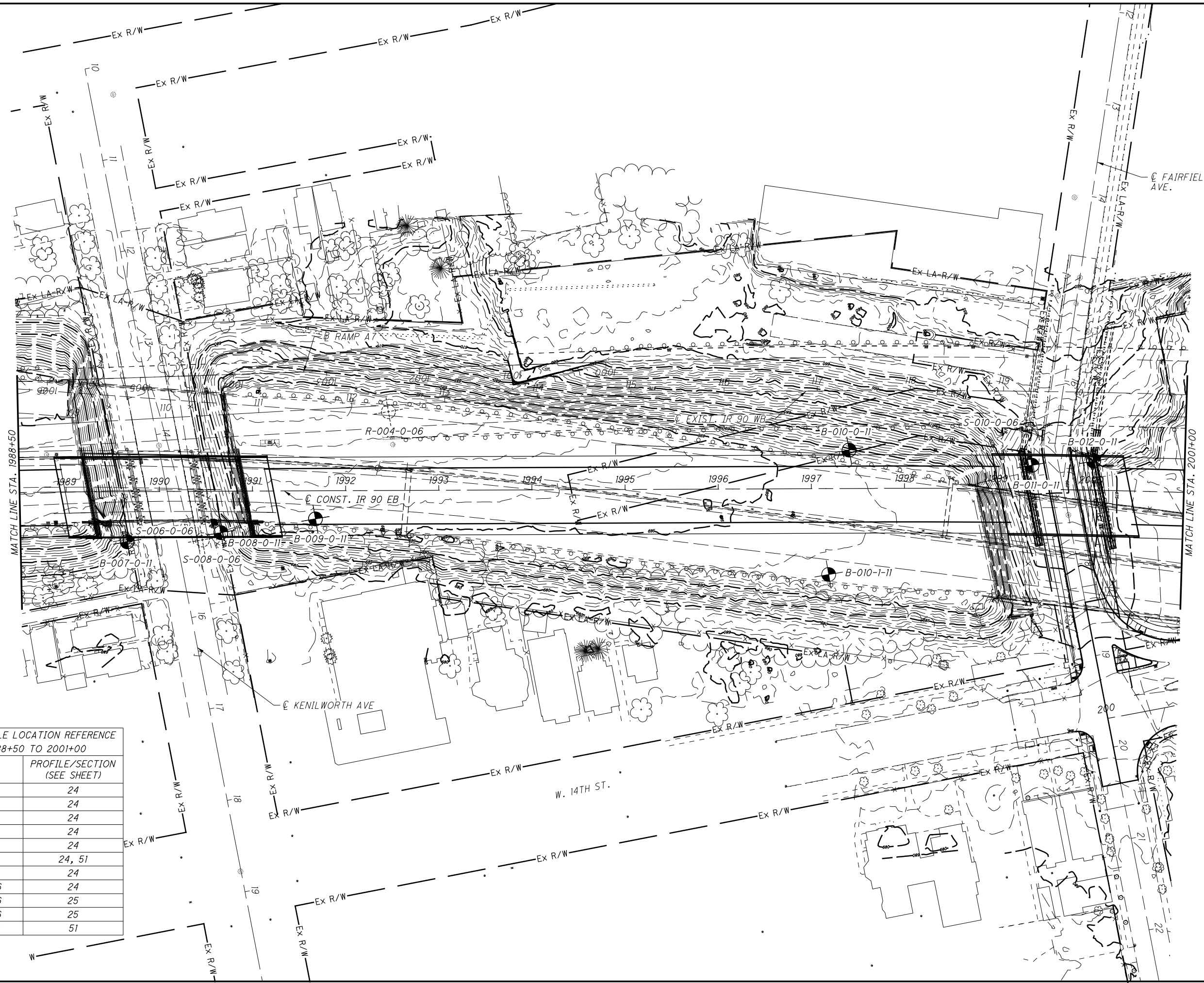
DRAWN: CMH  
CHECKED: JEP

**SOIL PROFILE  
STA. 1977+25 TO STA. 1988+50 IR 90 EB**

**CUY-90-14.90**

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BORING PROFILE LOCATION REFERENCE STA. 1988+50 TO 2001+00	
BORING ID	PROFILE/SECTION (SEE SHEET)
B-007-0-11	24
B-008-0-11	24
B-009-0-11	24
B-010-0-11	24
B-010-1-11	24
B-011-0-11	24, 51
B-012-0-11	24
R-004-0-06	24
S-006-0-06	25
S-008-0-06	25
S-010-0-06	51



DRAWN CMH  
CHECKED JEP

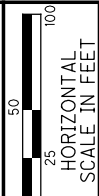
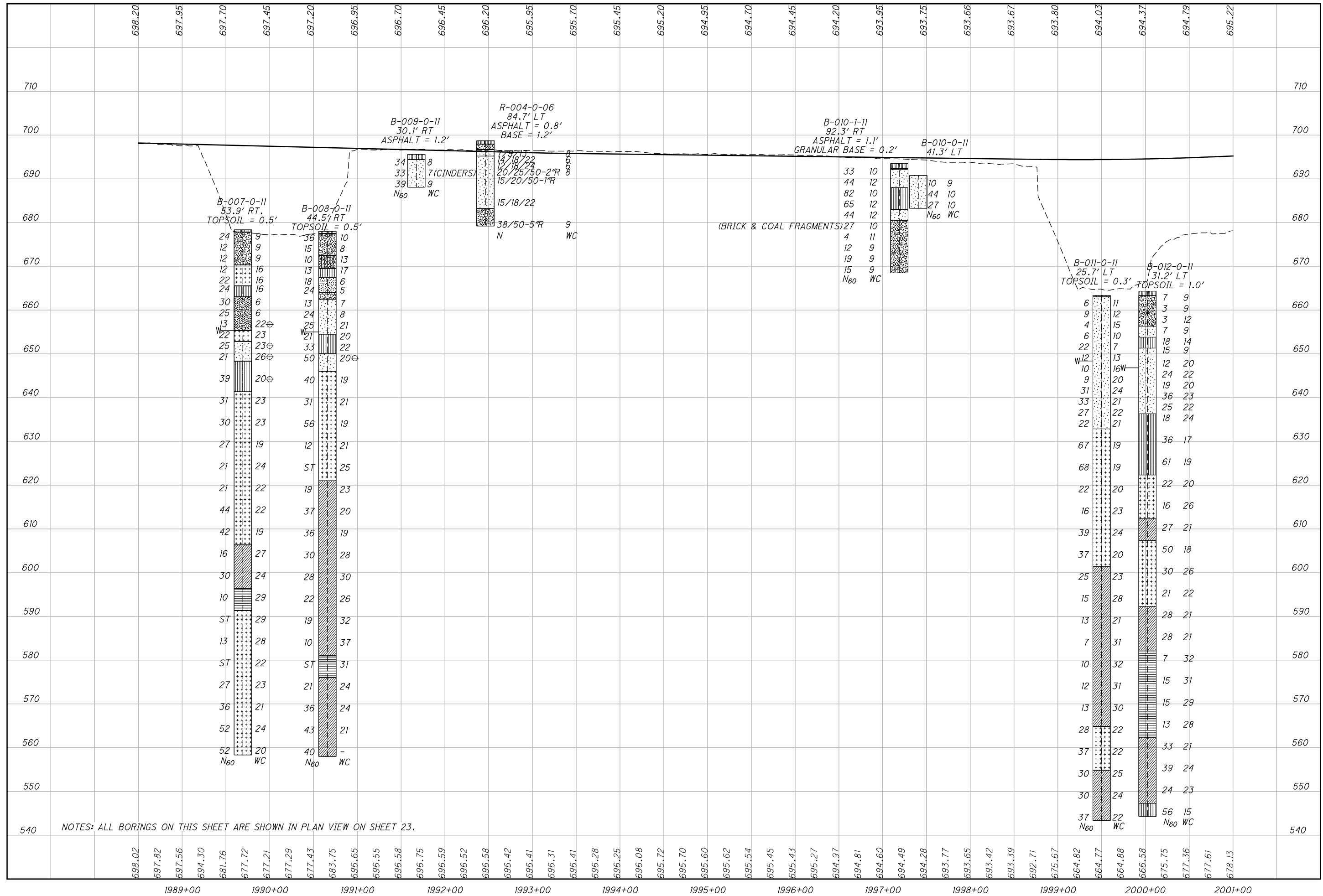
**SOIL PLAN**  
**STA. 1988+50 TO STA. 2001+00 IR 90 EB**

**CUY-90-14.90**





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DRAWN: CMH  
CHECKED: JEP

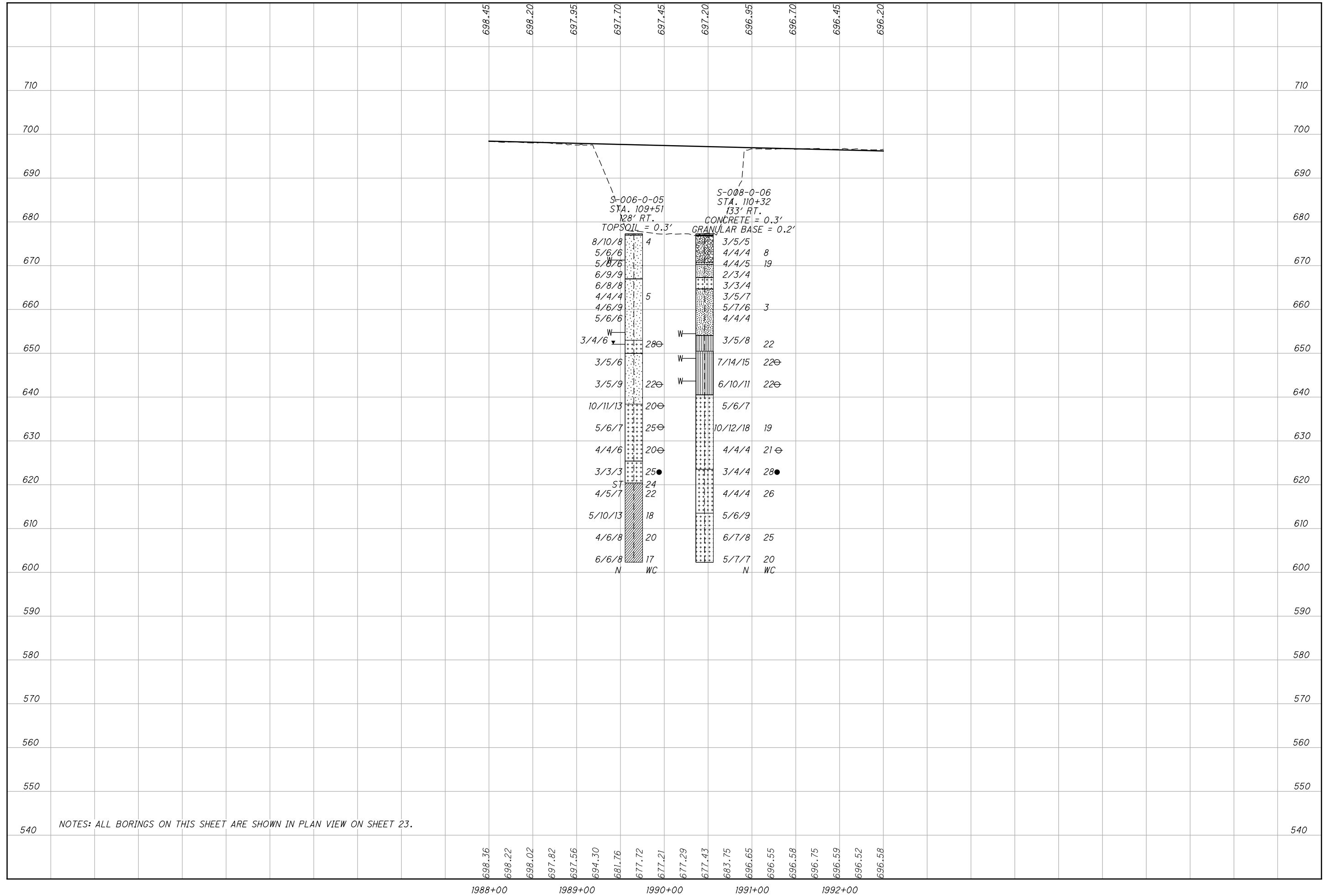
**SOIL PROFILE**  
**STA. 1988+50 TO STA. 2001+00 IR 90 EB**

**CUY-90-14.90**



NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 23.

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NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 23.

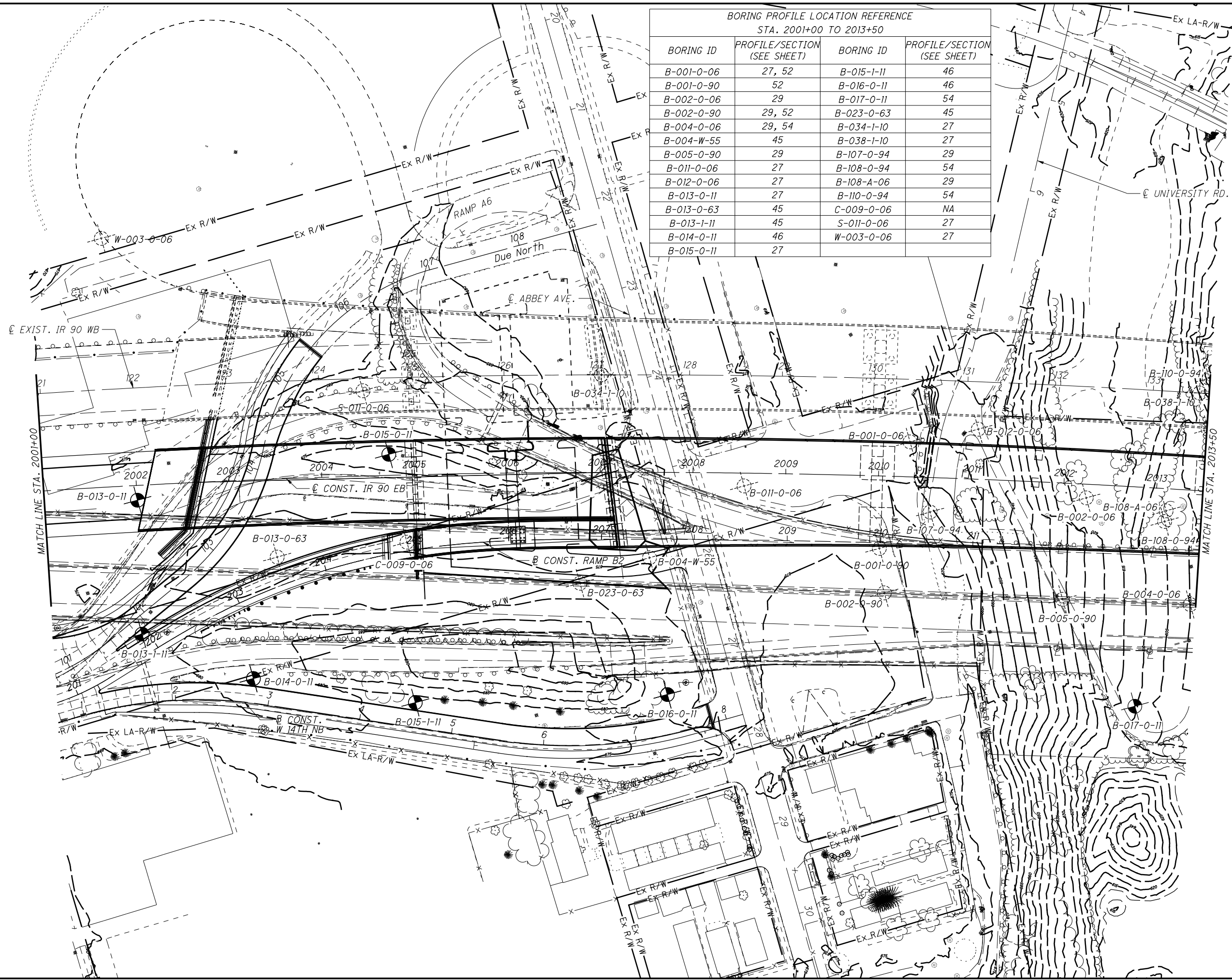
0 50 100  
25  
HORIZONTAL SCALE IN FEET  
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
STA. 1988+00 TO STA. 1991+50 IR 90 EB

CUY-90-14.90

25/67



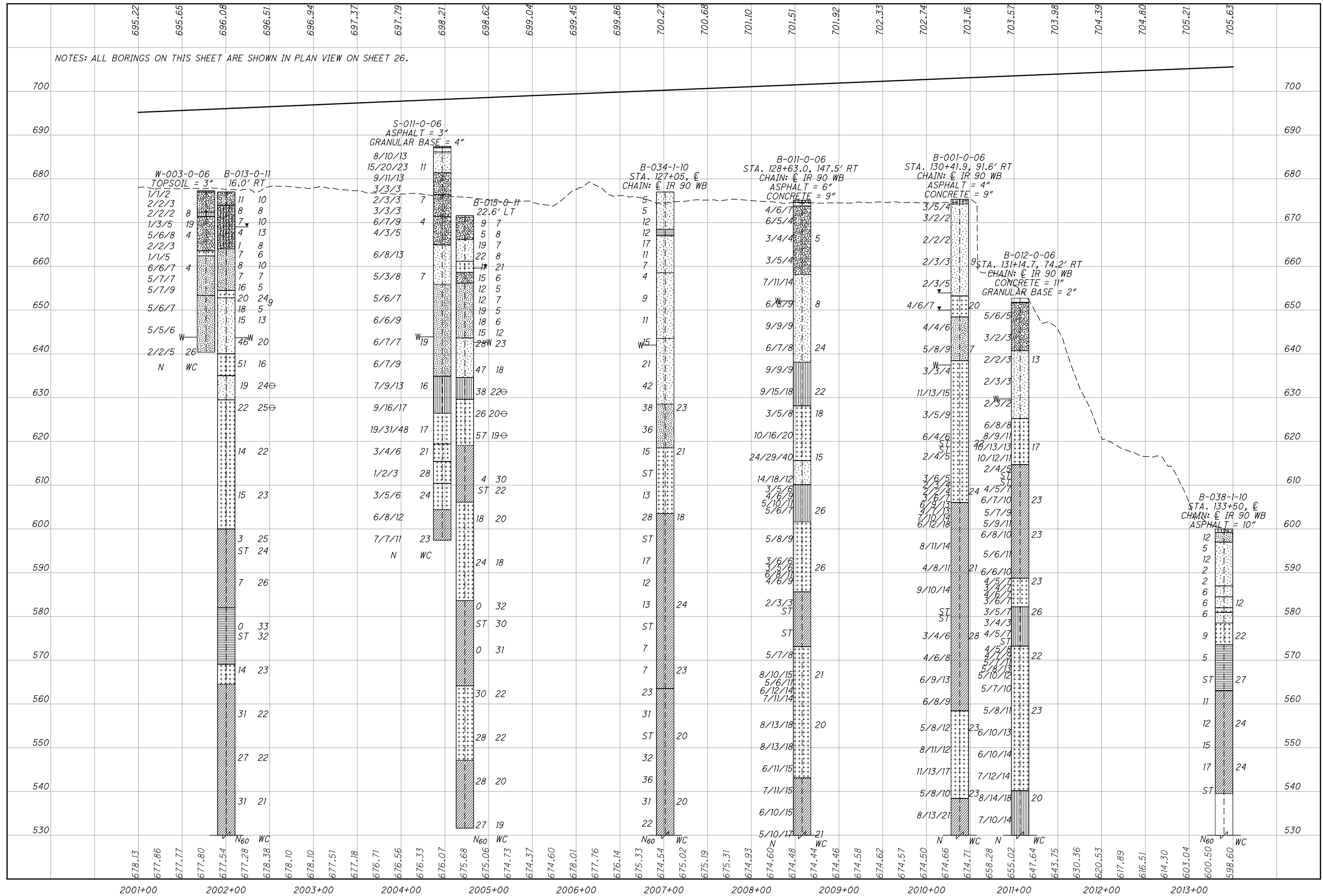


BORING PROFILE LOCATION REFERENCE STA. 2001+00 TO 2013+50			
BORING ID	PROFILE/SECTION (SEE SHEET)	BORING ID	PROFILE/SECTION (SEE SHEET)
B-001-0-06	27, 52	B-015-1-11	46
B-001-0-90	52	B-016-0-11	46
B-002-0-06	29	B-017-0-11	54
B-002-0-90	29, 52	B-023-0-63	45
B-004-0-06	29, 54	B-034-1-10	27
B-004-W-55	45	B-038-1-10	27
B-005-0-90	29	B-107-0-94	29
B-011-0-06	27	B-108-0-94	54
B-012-0-06	27	B-108-A-06	29
B-013-0-11	27	B-110-0-94	54
B-013-0-63	45	C-009-0-06	NA
B-013-1-11	45	S-011-0-06	27
B-014-0-11	46	W-003-0-06	27
B-015-0-11	27		



DRAWN CMH  
 CHECKED JEP  
**SOIL PLAN**  
**STA. 2001+00 TO STA. 2013+50 IR 90 EB**

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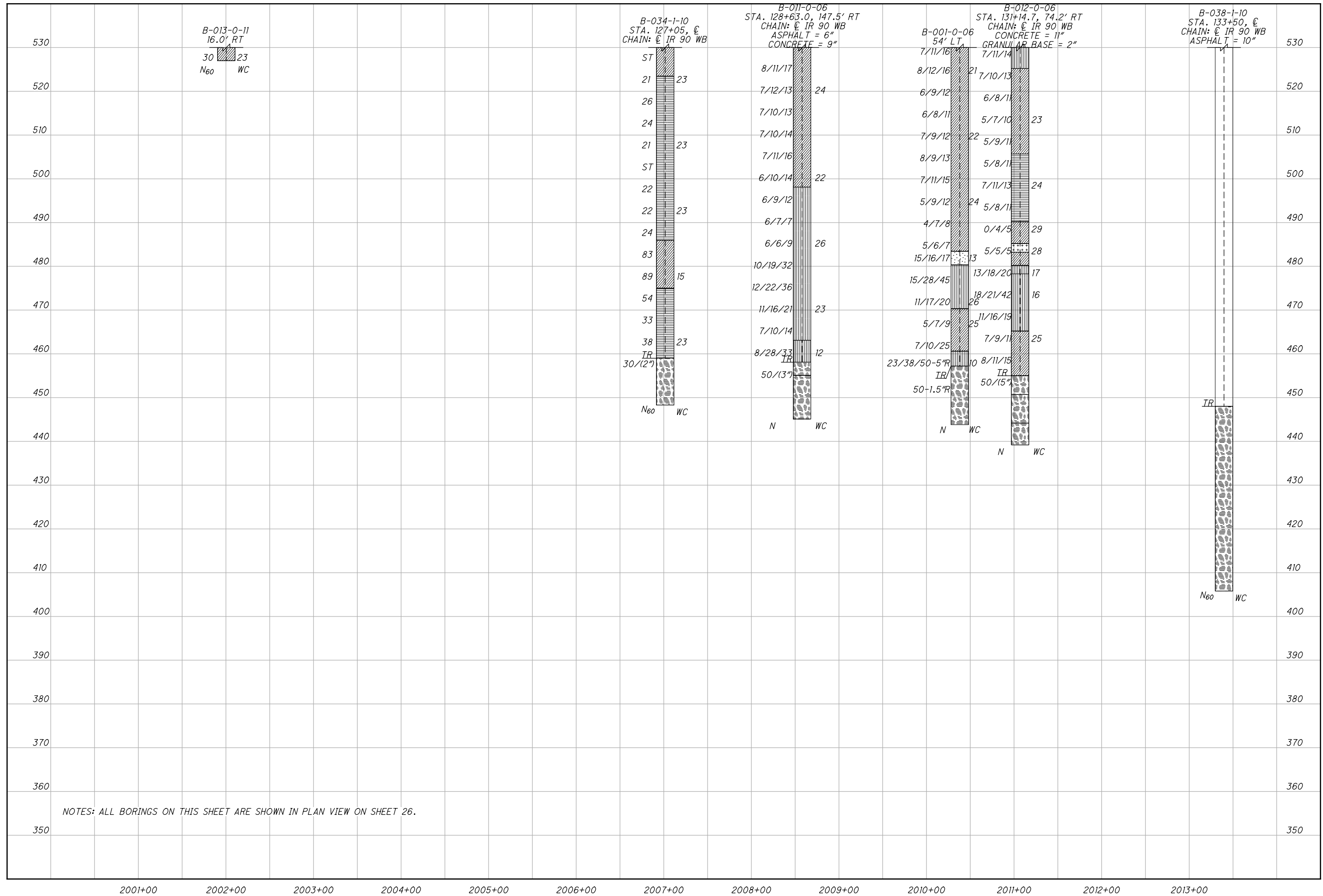
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
STA. 2001+00 TO STA. 2013+50 IR 90 EB

CUY-90-14.90



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NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 26.

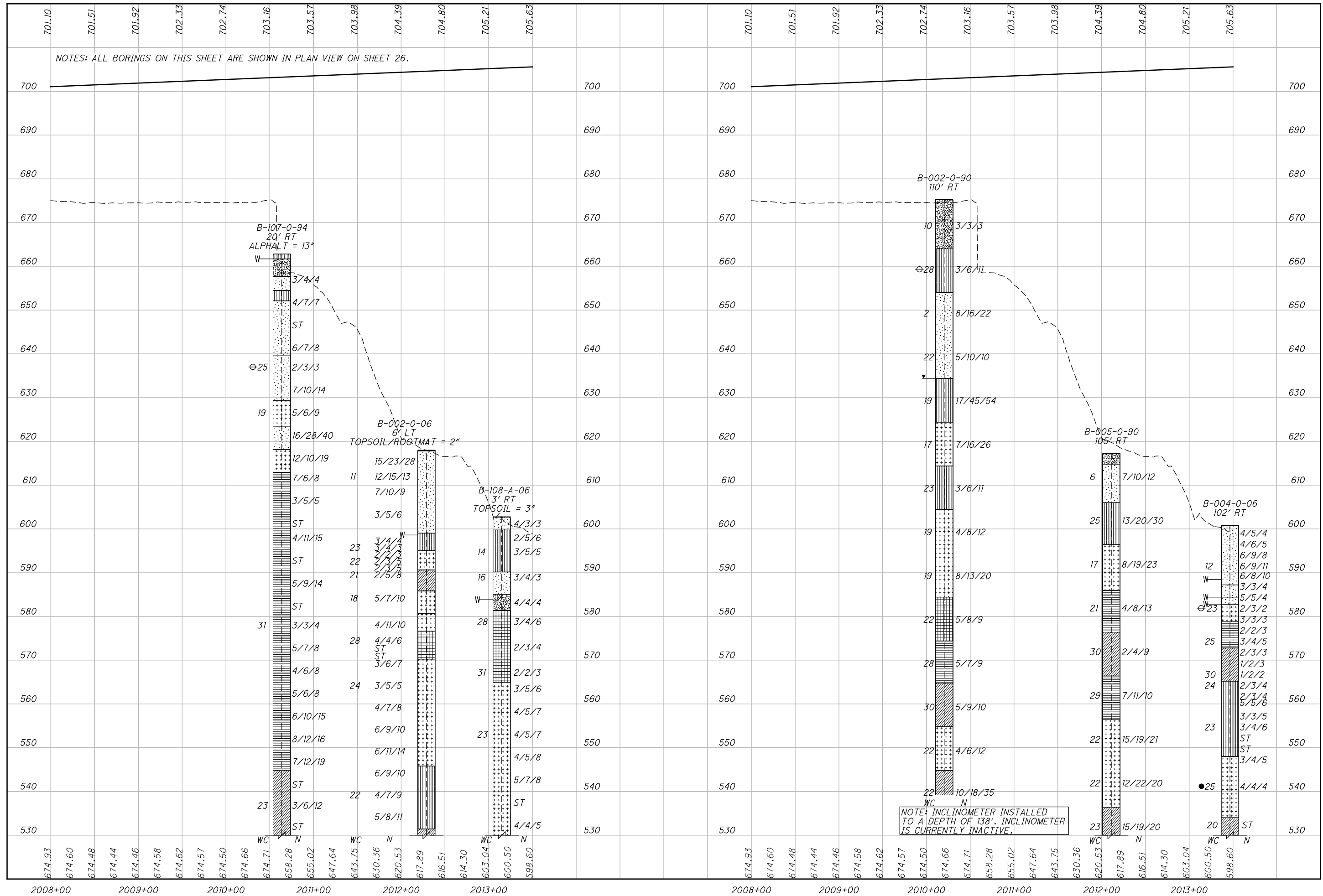


**SOIL PROFILE**  
**STA. 2001+00 TO STA. 2013+50 IR 90 EB**

CUY-90-14.90



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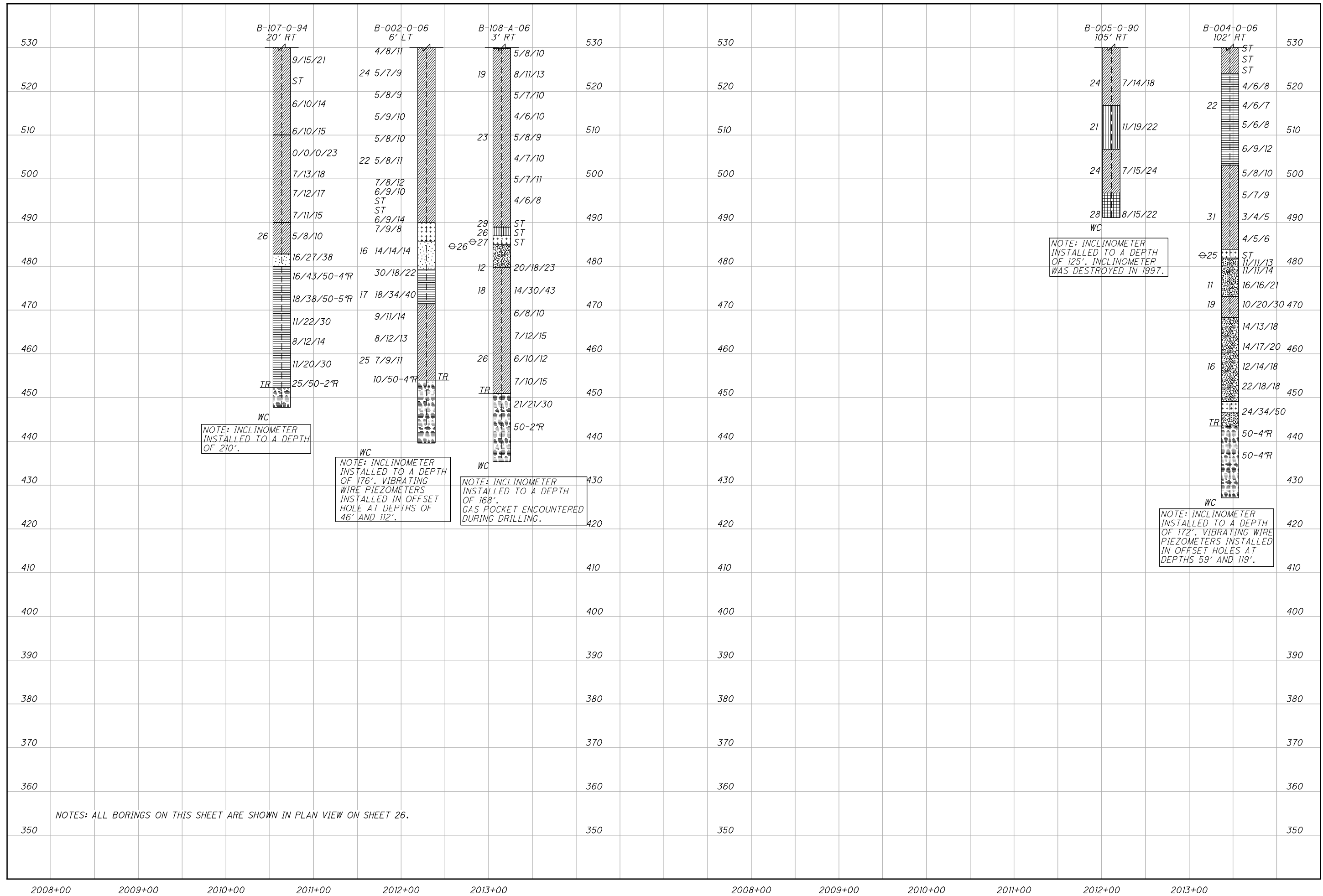


0 50 100  
 DRAWN CMH  
 CHECKED JEP  
 HORIZONTAL SCALE IN FEET

**SOIL PROFILE**  
**STA. 2008+00 TO STA. 2013+50 IR 90 EB**

**CUY-90-14.90**

F:\CUIY\B2119\geotechnical\sheets\B2119\F008.dgn 7/20/2012 7:19:25 PM che

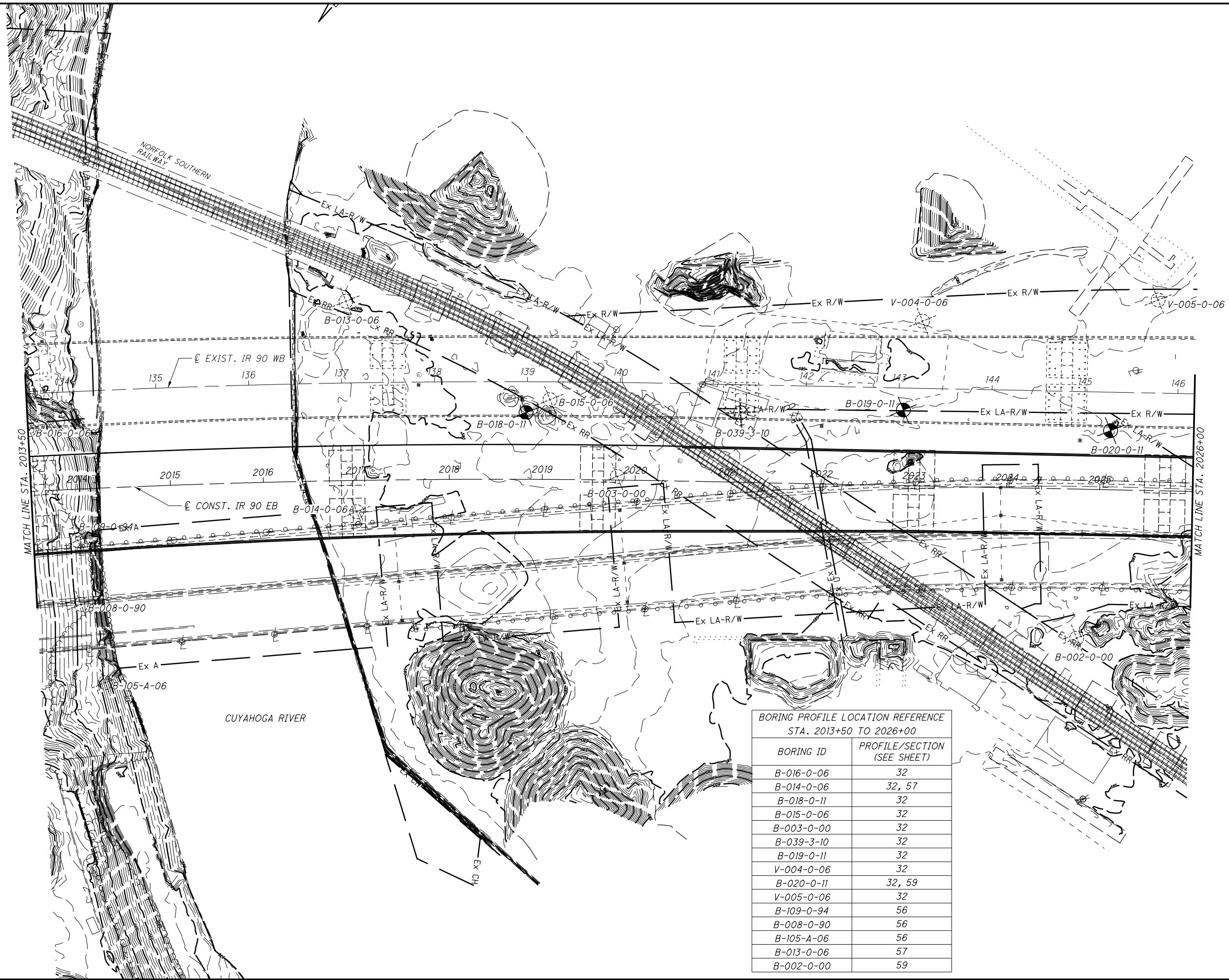


NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 26.



SOIL PROFILE  
STA. 2008+00 TO STA. 2013+50 IR 90 EB

CUY-90-14.90



BORING PROFILE LOCATION REFERENCE  
STA. 2013+50 TO 2026+00

BORING ID	PROFILE/SECTION (SEE SHEET)
B-016-0-06	32
B-014-0-06	32, 57
B-018-0-11	32
B-015-0-06	32
B-003-0-00	32
B-039-3-10	32
B-019-0-11	32
V-004-0-06	32
B-020-0-11	32, 59
V-005-0-06	32
B-109-0-94	56
B-008-0-90	56
B-105-A-06	56
B-013-0-06	57
B-002-0-00	59



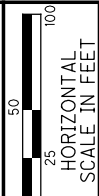
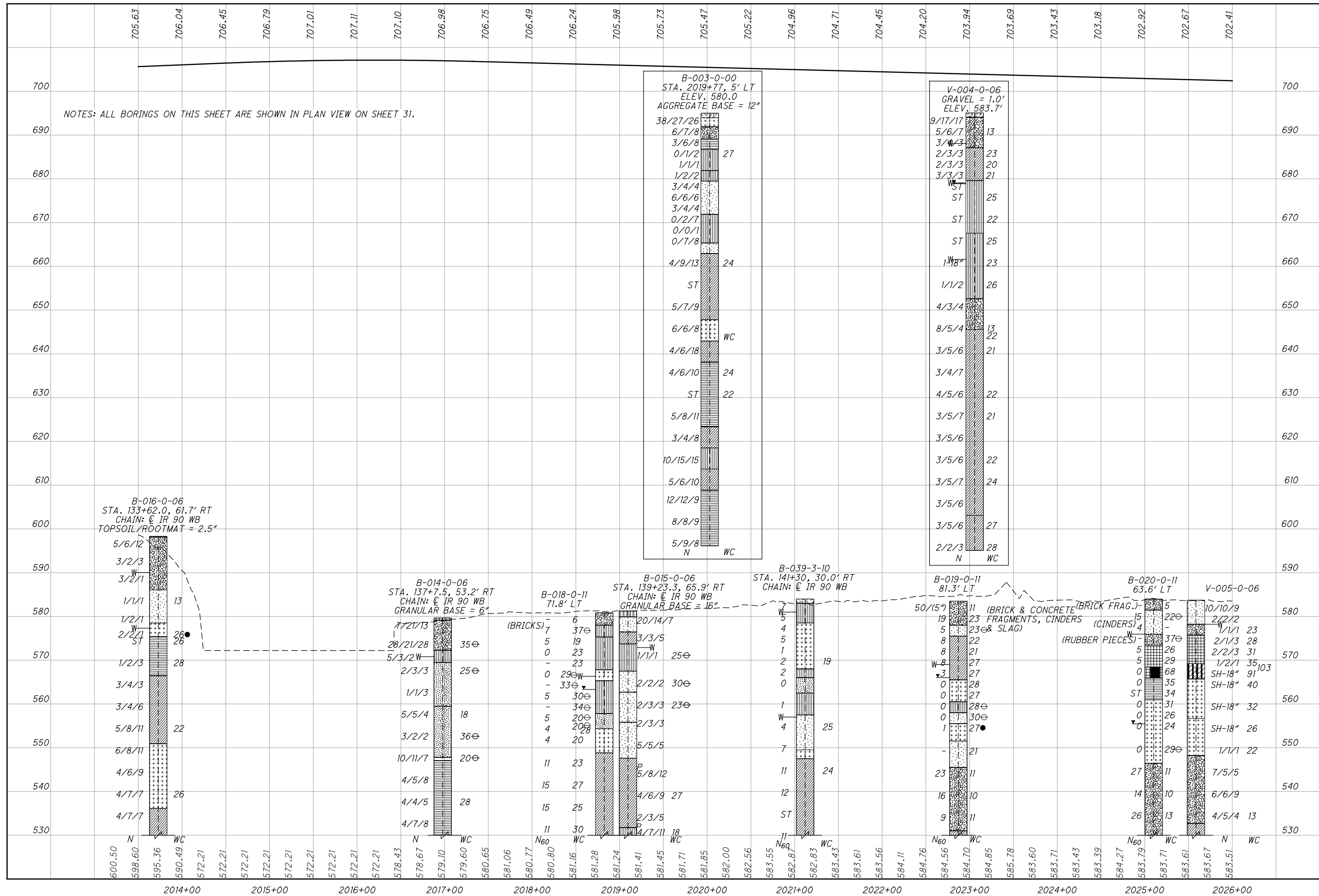
DRAWN  
CMH  
CHECKED  
JEP

**SOIL PLAN**  
**STA. 2013+50 TO STA. 2026+00 IR 90 EB**

**CUY-90-14.90**



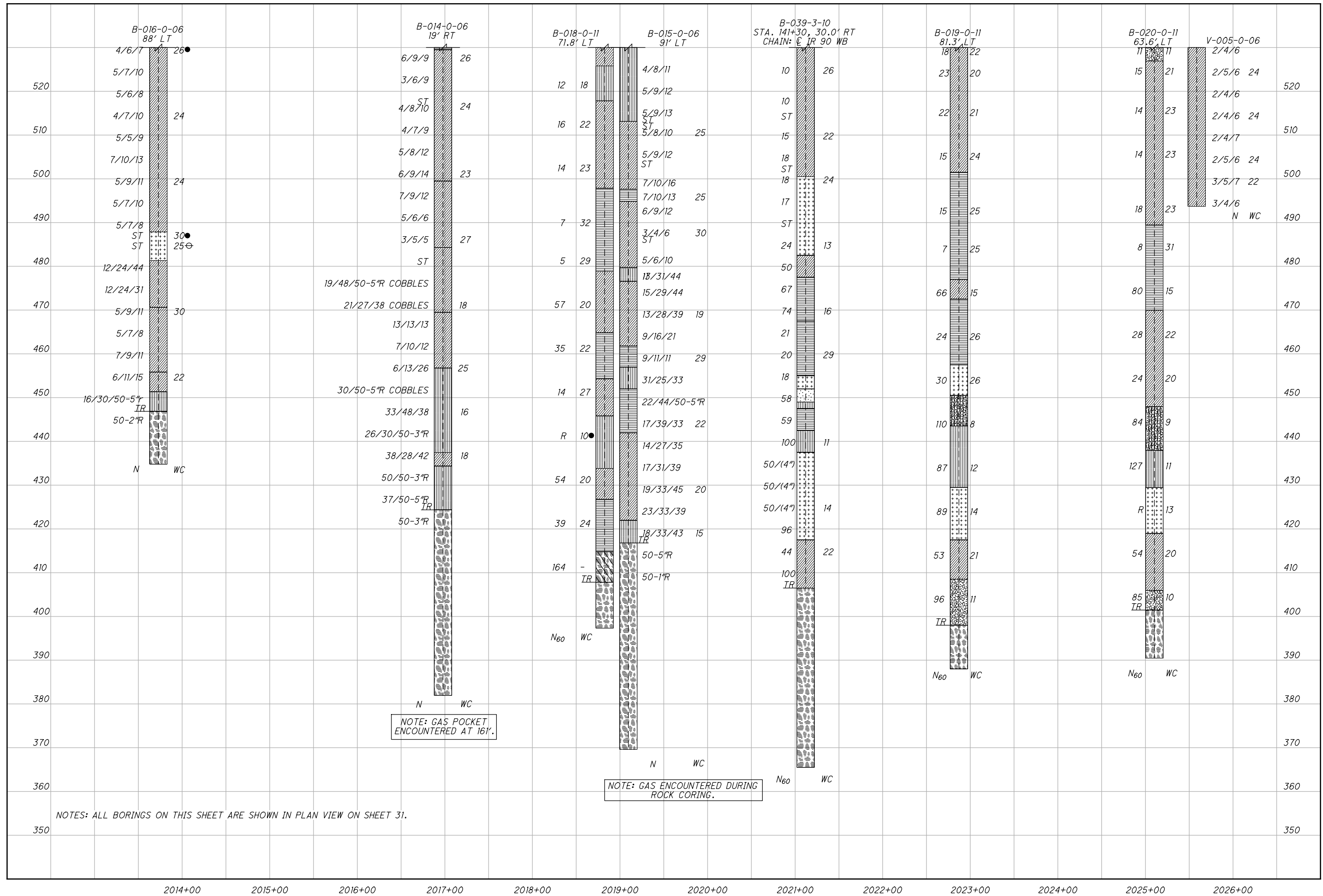




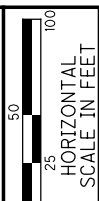
DRAWN CMH  
 CHECKED JEP

**SOIL PROFILE**  
**STA. 2013+50 TO STA. 2026+00 IR 90 EB**

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NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 31.



DRAWN CMH CHECKED JEP

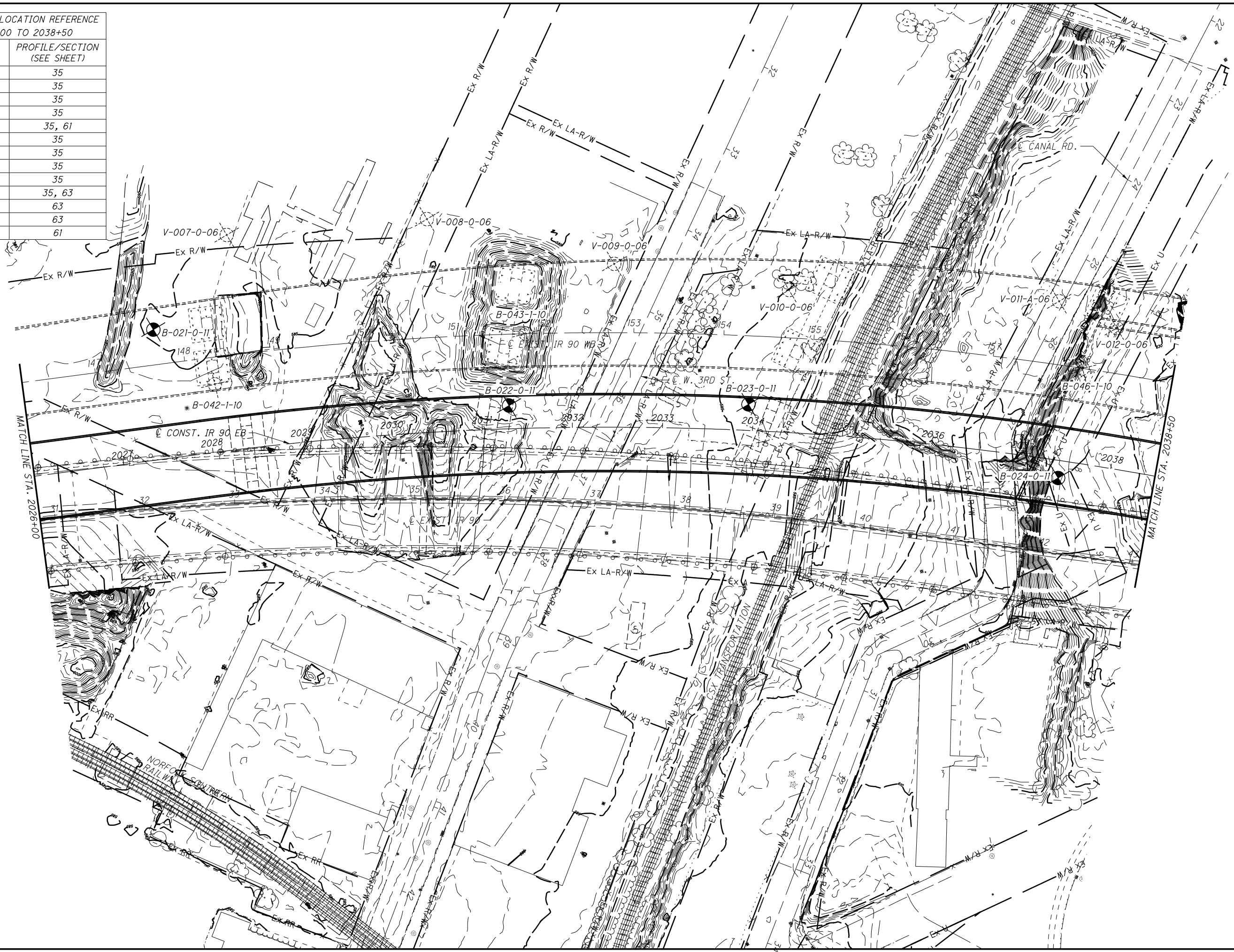
SOIL PROFILE STA. 2013+50 TO STA. 2026+00 IR 90 EB

CUY-90-14.90



BORING PROFILE LOCATION REFERENCE  
STA. 2026+00 TO 2038+50

BORING ID	PROFILE/SECTION (SEE SHEET)
V-007-0-06	35
B-021-0-11	35
B-042-1-10	35
V-008-0-06	35
B-022-0-11	35, 61
V-009-0-06	35
B-023-0-11	35
V-010-0-06	35
V-011-A-06	35
B-024-0-11	35, 63
V-012-0-06	63
B-046-1-10	63
B-043-1-10	61

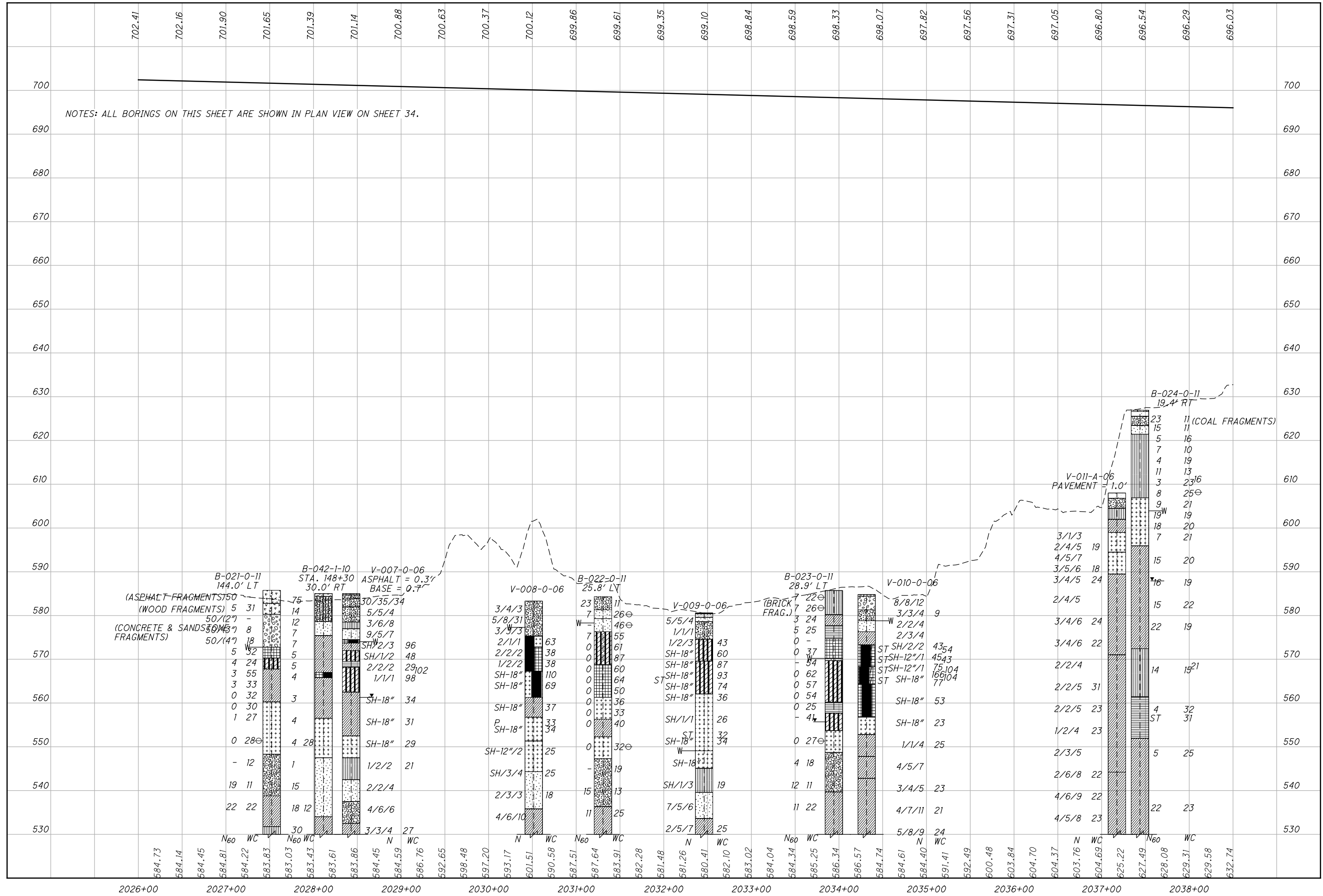


  
 DRAWN: CMH  
 CHECKED: JEP

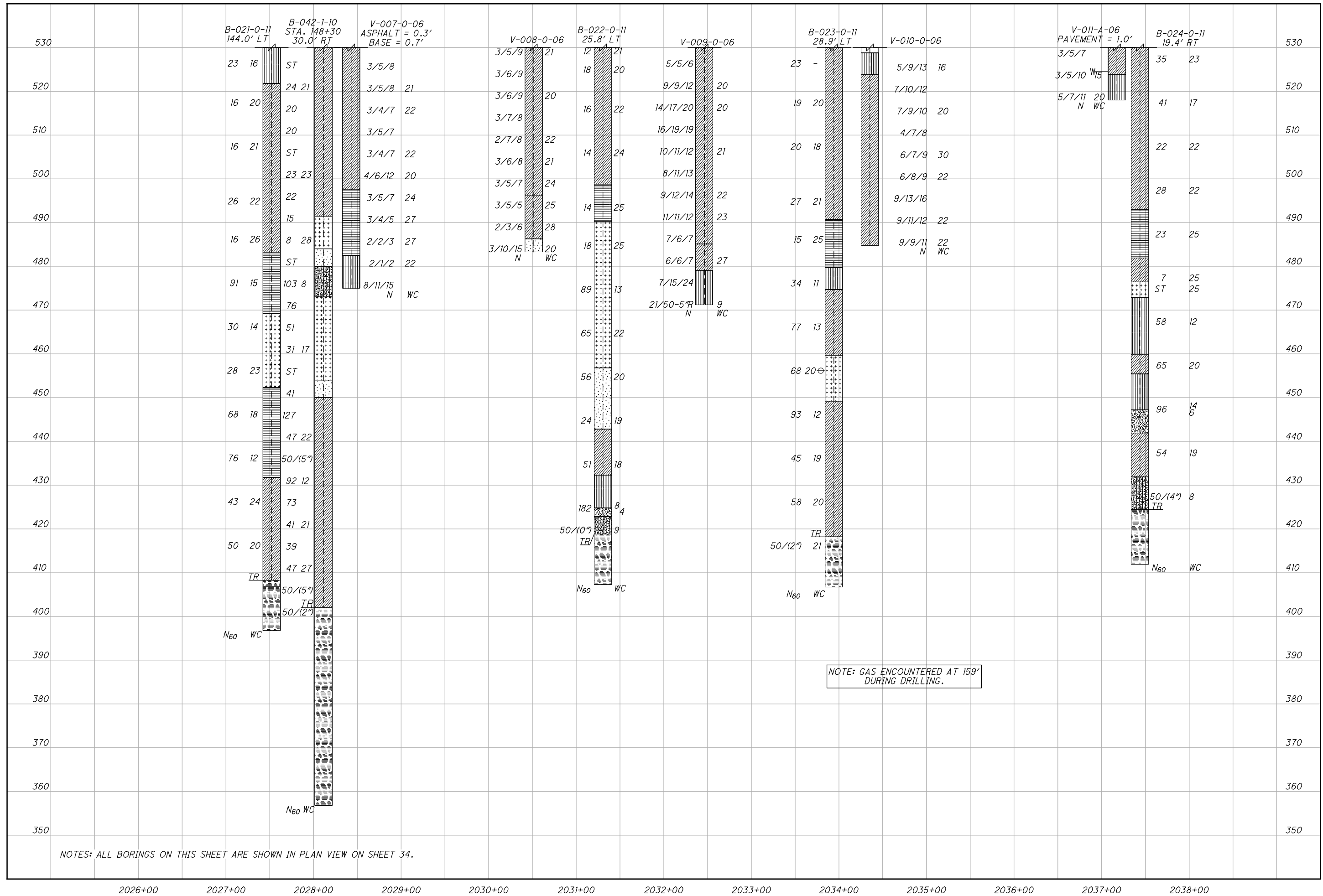
**SOIL PLAN**  
**STA. 2026+00 TO 2038+50 IR 90 EB**

**CUY-90-14.90**



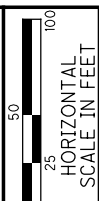
SOIL PROFILE  
STA. 2026+00 TO STA. 2038+50 IR 90 EB

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NOTE: GAS ENCOUNTERED AT 159' DURING DRILLING.

NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 34.



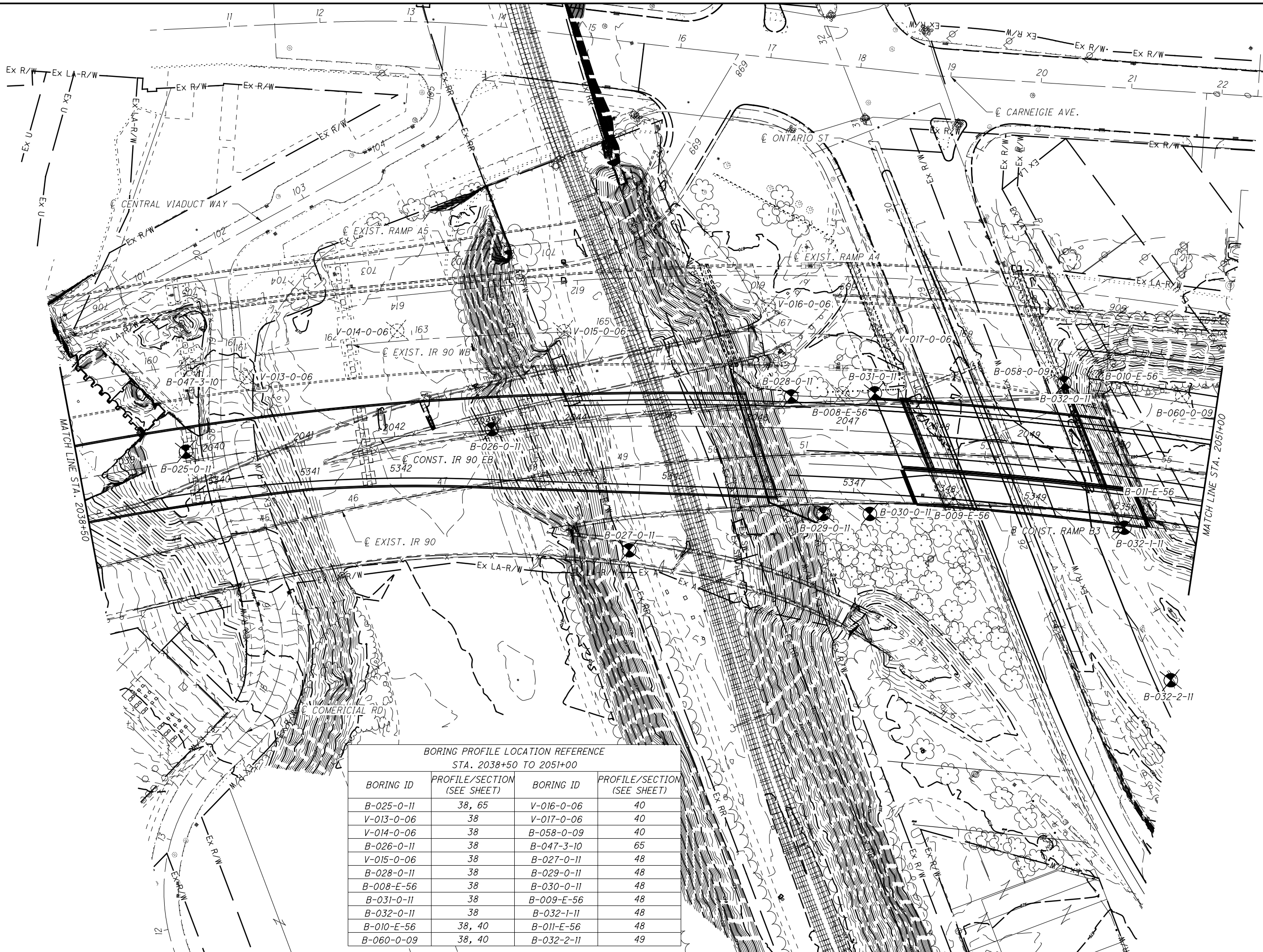
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
STA. 2026+00 TO STA. 2038+50 IR 90 EB

CUY-90-14.90



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BORING PROFILE LOCATION REFERENCE  
STA. 2038+50 TO 2051+00

BORING ID	PROFILE/SECTION (SEE SHEET)	BORING ID	PROFILE/SECTION (SEE SHEET)
B-025-0-11	38, 65	V-016-0-06	40
V-013-0-06	38	V-017-0-06	40
V-014-0-06	38	B-058-0-09	40
B-026-0-11	38	B-047-3-10	65
V-015-0-06	38	B-027-0-11	48
B-028-0-11	38	B-029-0-11	48
B-008-E-56	38	B-030-0-11	48
B-031-0-11	38	B-009-E-56	48
B-010-E-56	38, 40	B-032-1-11	48
B-060-0-09	38, 40	B-011-E-56	48
		B-032-2-11	49

0 25 50 100  
HORIZONTAL  
SCALE IN FEET

DRAWN  
CMH

CHECKED  
JEP

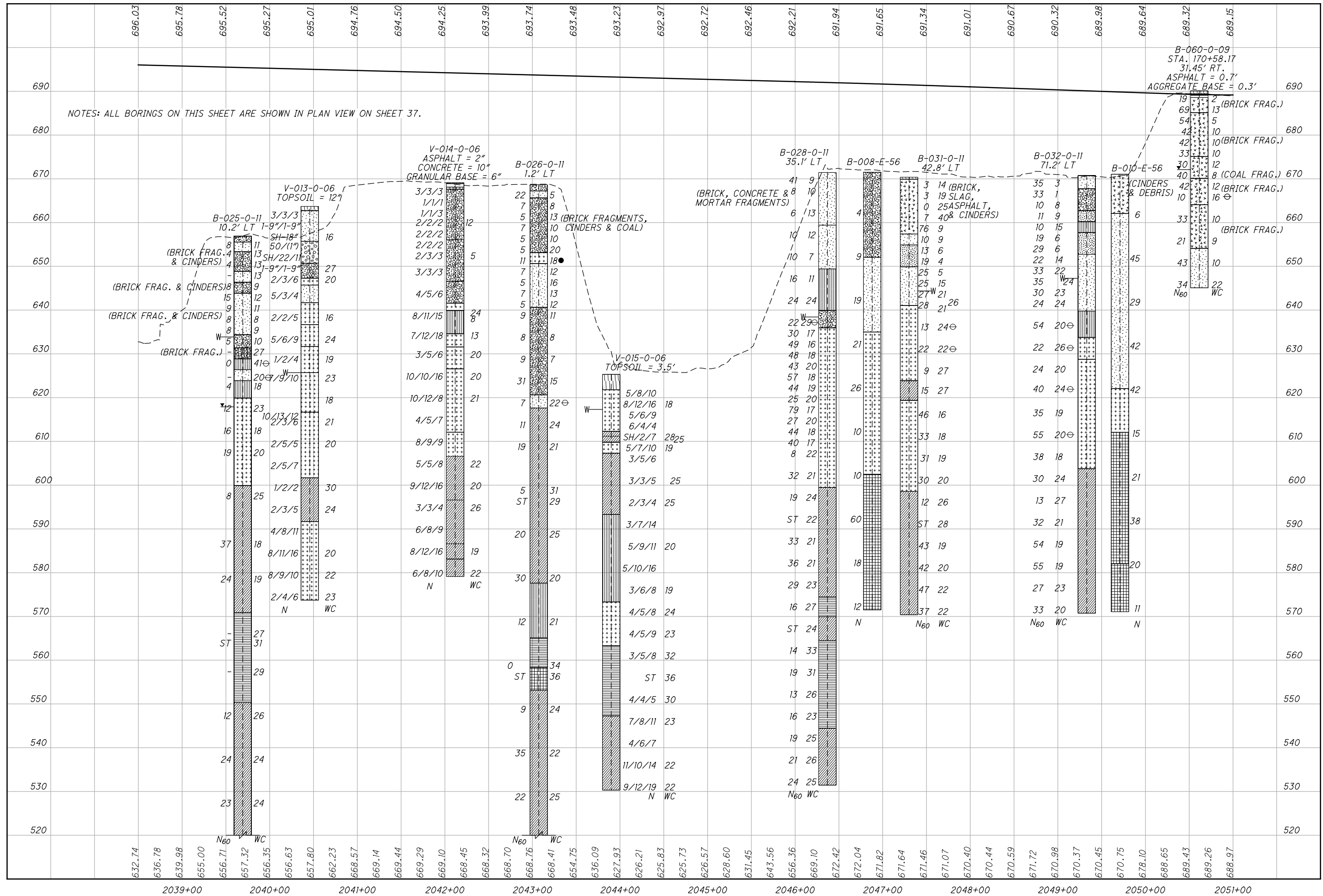
**SOIL PLAN**

**STA. 2038+50 TO STA. 2051+00 IR 90 EB**

**CUY-90-14.90**

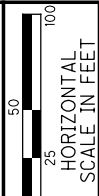
37/67

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NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 37.

B-060-0-09  
STA. 170+58.17  
31.45' RT.  
ASPHALT = 0.7'  
AGGREGATE BASE = 0.3'



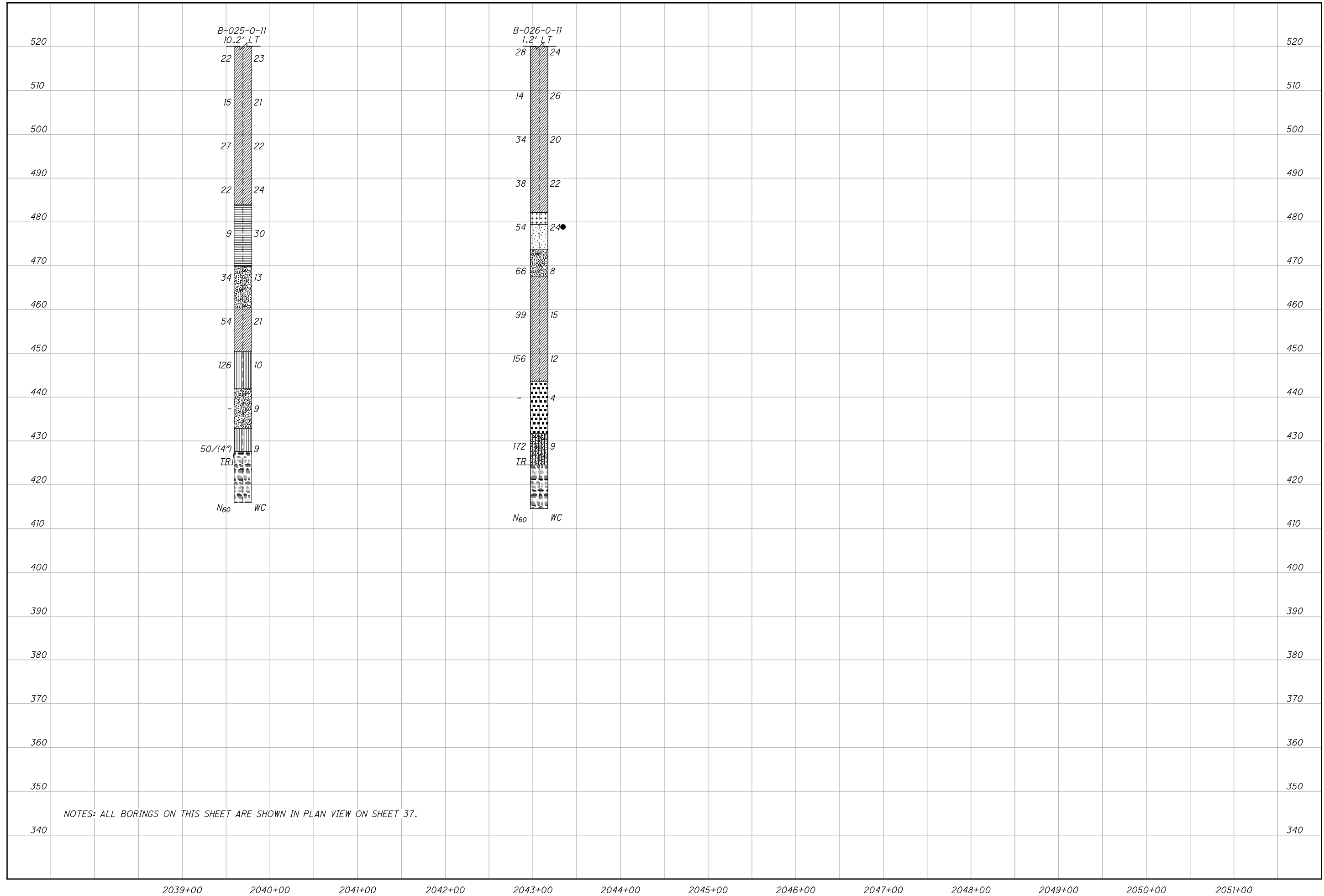
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
STA. 2038+50 TO STA. 2051+00 IR 90 EB

CUY-90-14.90



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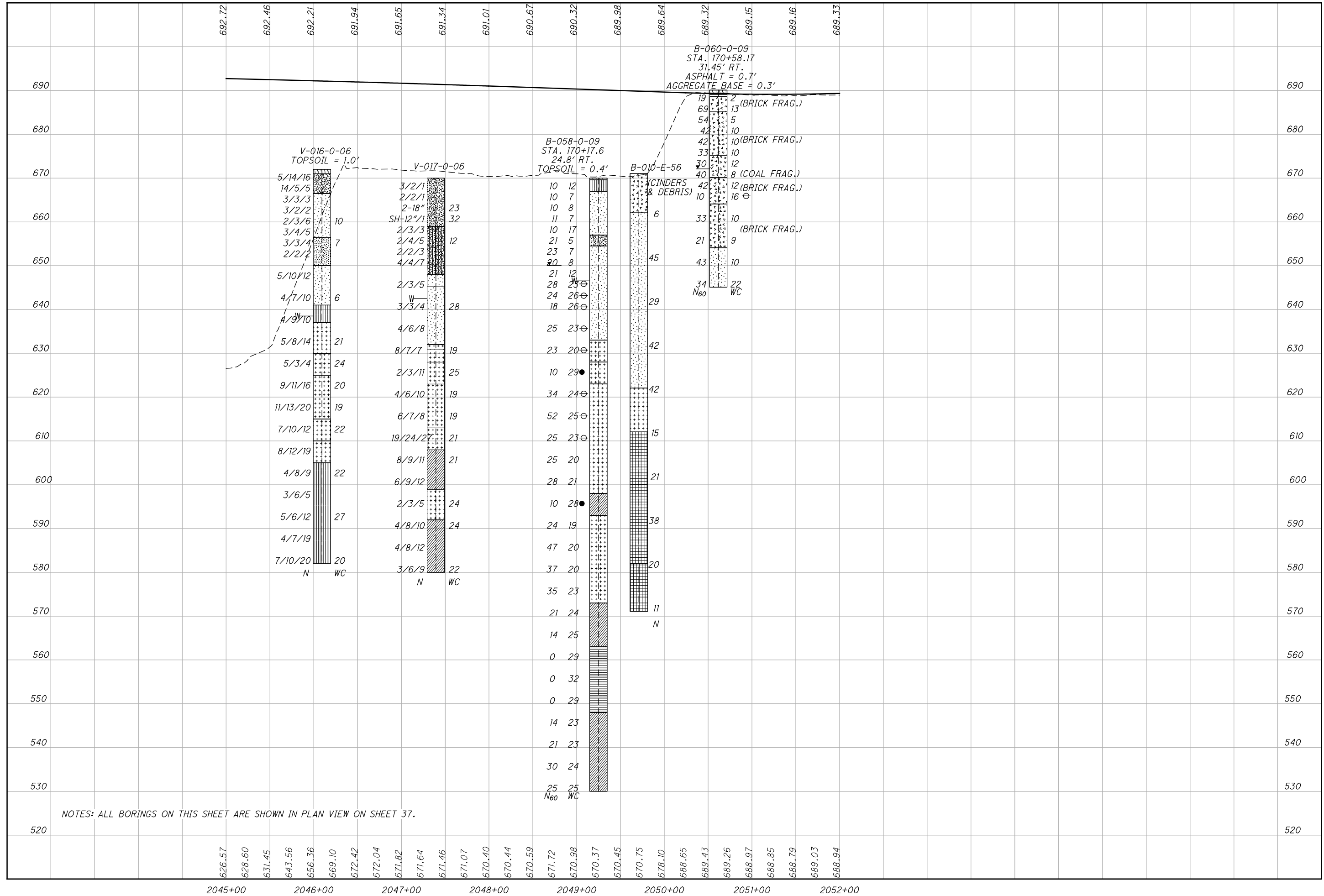
DRAWN: CMH  
CHECKED: JEP

**SOIL PROFILE**  
**STA. 2038+50 TO STA. 2051+00 IR 90 EB**

**CUY-90-14.90**





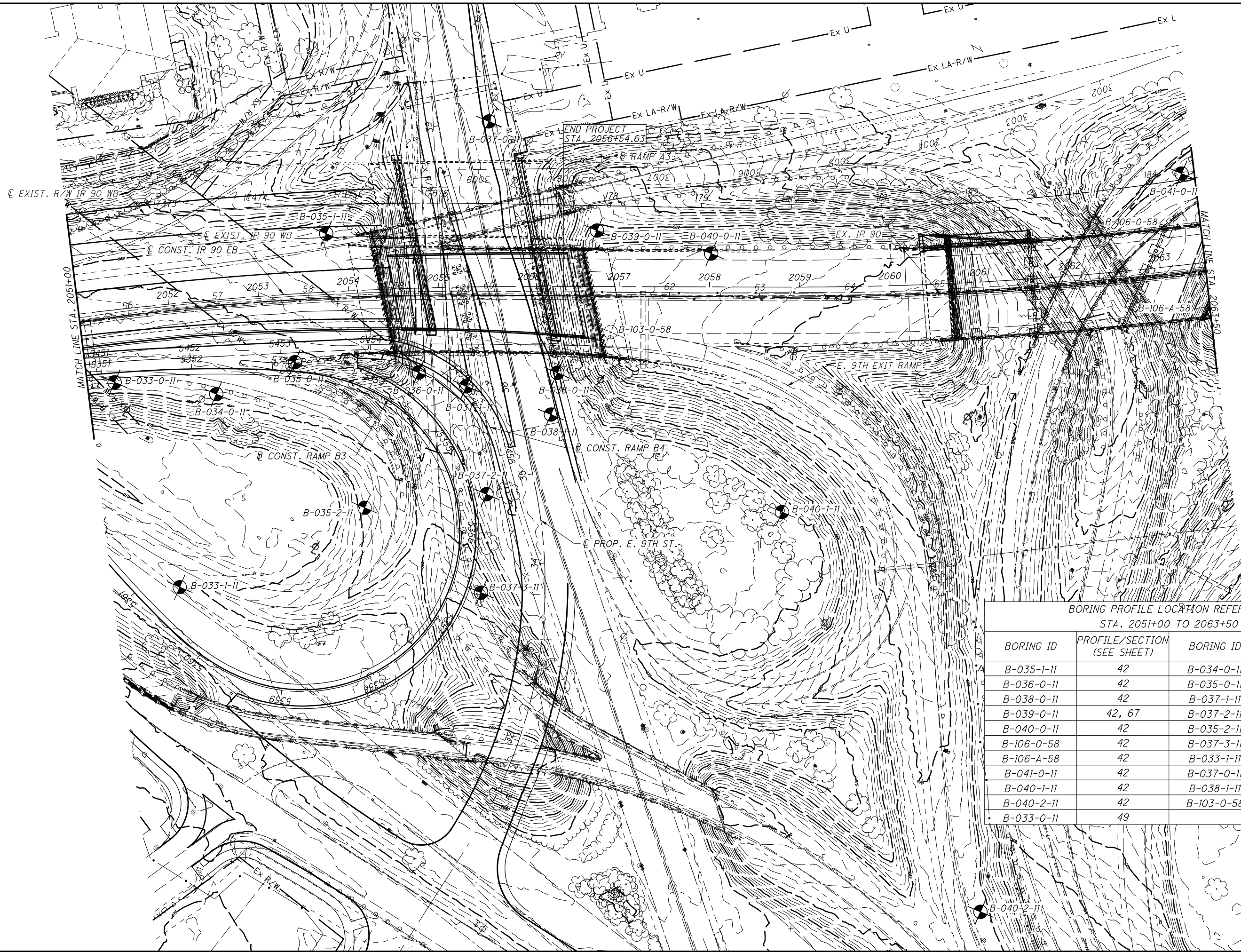


DRAWN CMH  
 CHECKED JEP

**SOIL PROFILE**  
**STA. 2045+00 TO STA. 2052+00 IR 90 EB**

NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 37.

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BORING PROFILE LOCATION REFERENCE  
STA. 2051+00 TO 2063+50

BORING ID	PROFILE/SECTION (SEE SHEET)	BORING ID	PROFILE/SECTION (SEE SHEET)
B-035-1-11	42	B-034-0-11	49
B-036-0-11	42	B-035-0-11	49
B-038-0-11	42	B-037-1-11	49
B-039-0-11	42, 67	B-037-2-11	49
B-040-0-11	42	B-035-2-11	49
B-106-0-58	42	B-037-3-11	49
B-106-A-58	42	B-033-1-11	49
B-041-0-11	42	B-037-0-11	50
B-040-1-11	42	B-038-1-11	50
B-040-2-11	42	B-103-0-58	67
B-033-0-11	49		

0 50 100  
25  
HORIZONTAL  
SCALE IN FEET

DRAWN

CMH

CHECKED

JEP

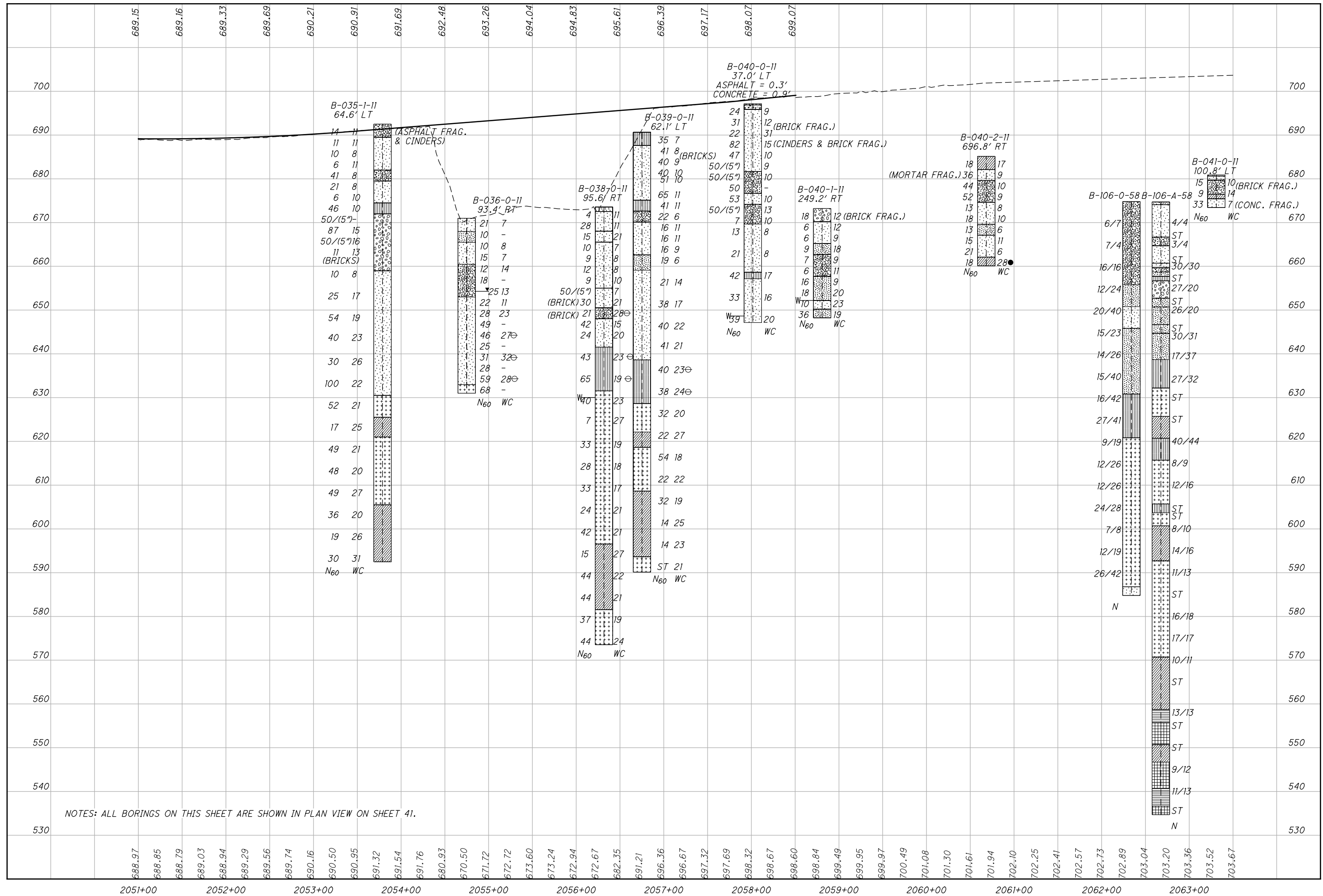
SOIL PLAN

STA. 2051+00 TO STA. 2063+50 IR 90 EB

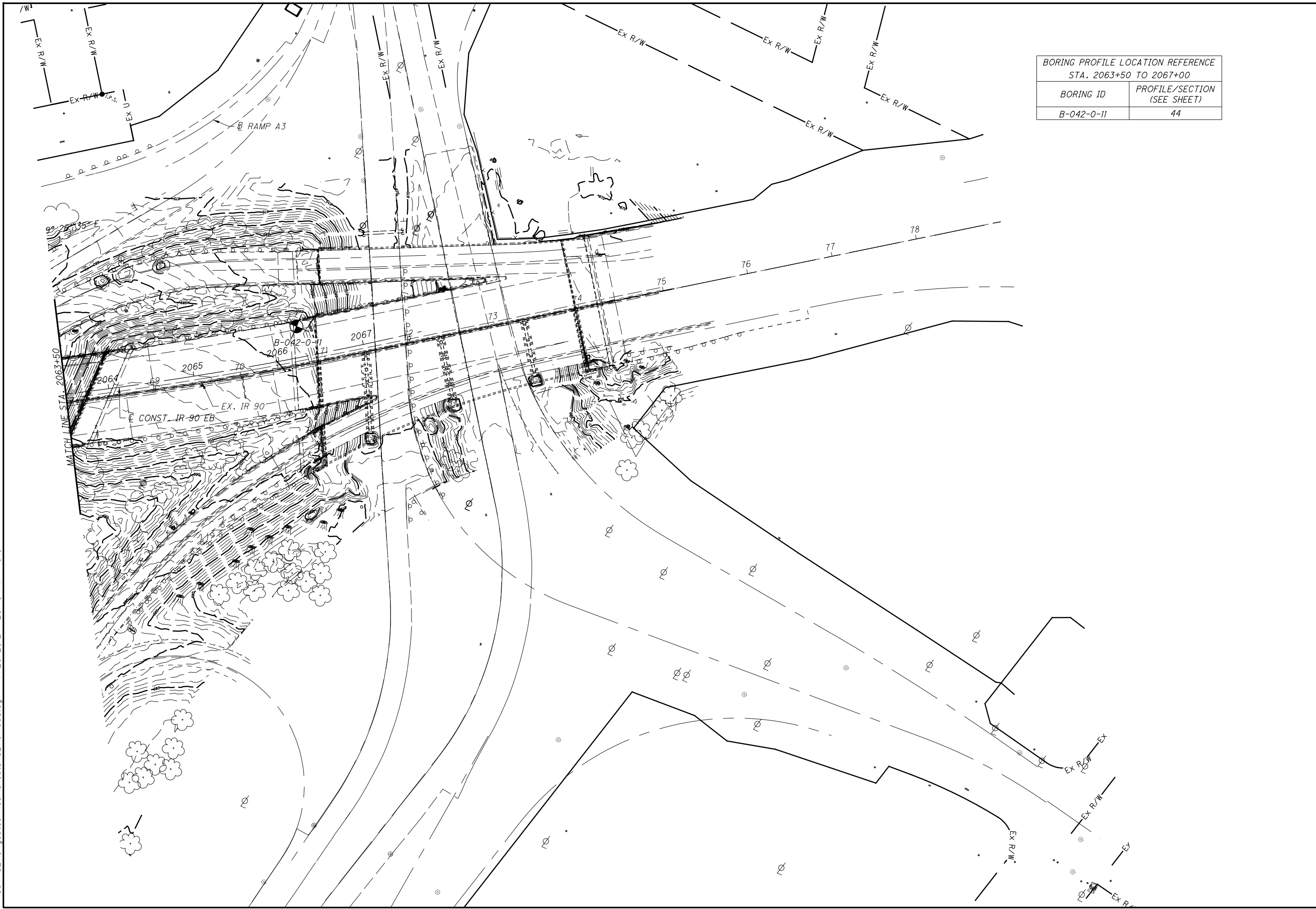
CUY-90-14.90

41

/ 67



**SOIL PROFILE**  
**STA. 2051+00 TO STA. 2063+50 IR 90 EB**



BORING PROFILE LOCATION REFERENCE  
STA. 2063+50 TO 2067+00

BORING ID	PROFILE/SECTION (SEE SHEET)
B-042-0-11	44

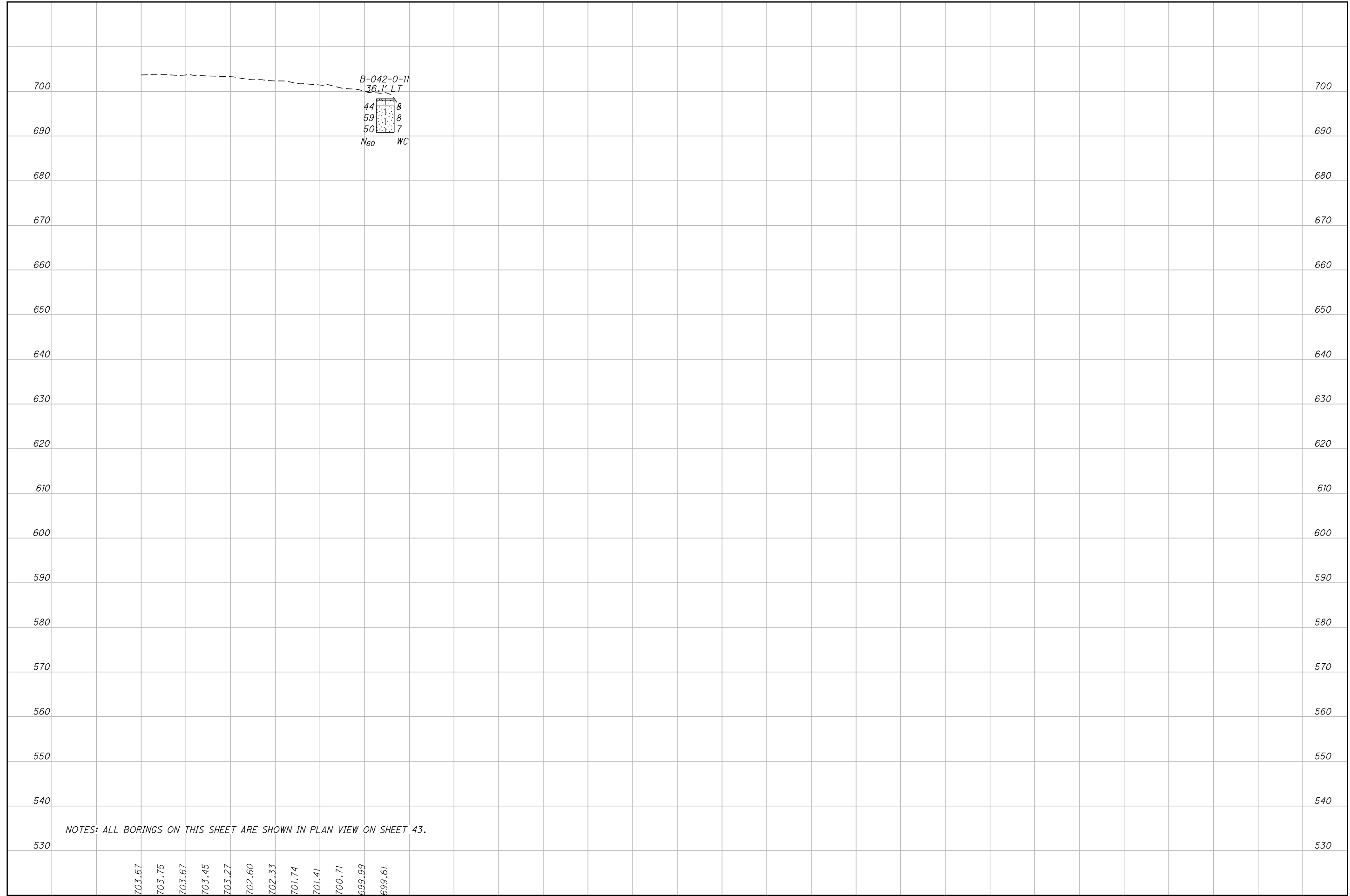


DRAWN CMH  
CHECKED JEP

**SOIL PLAN**  
**STA. 2063+50 TO STA. 2067+00 IR 90 EB**

**CUY-90-14.90**





NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 43.

703.67  
 703.75  
 703.67  
 703.45  
 703.27  
 702.60  
 702.33  
 701.74  
 701.41  
 700.71  
 599.99  
 599.61

2064+00                      2065+00                      2066+00                      2067+00

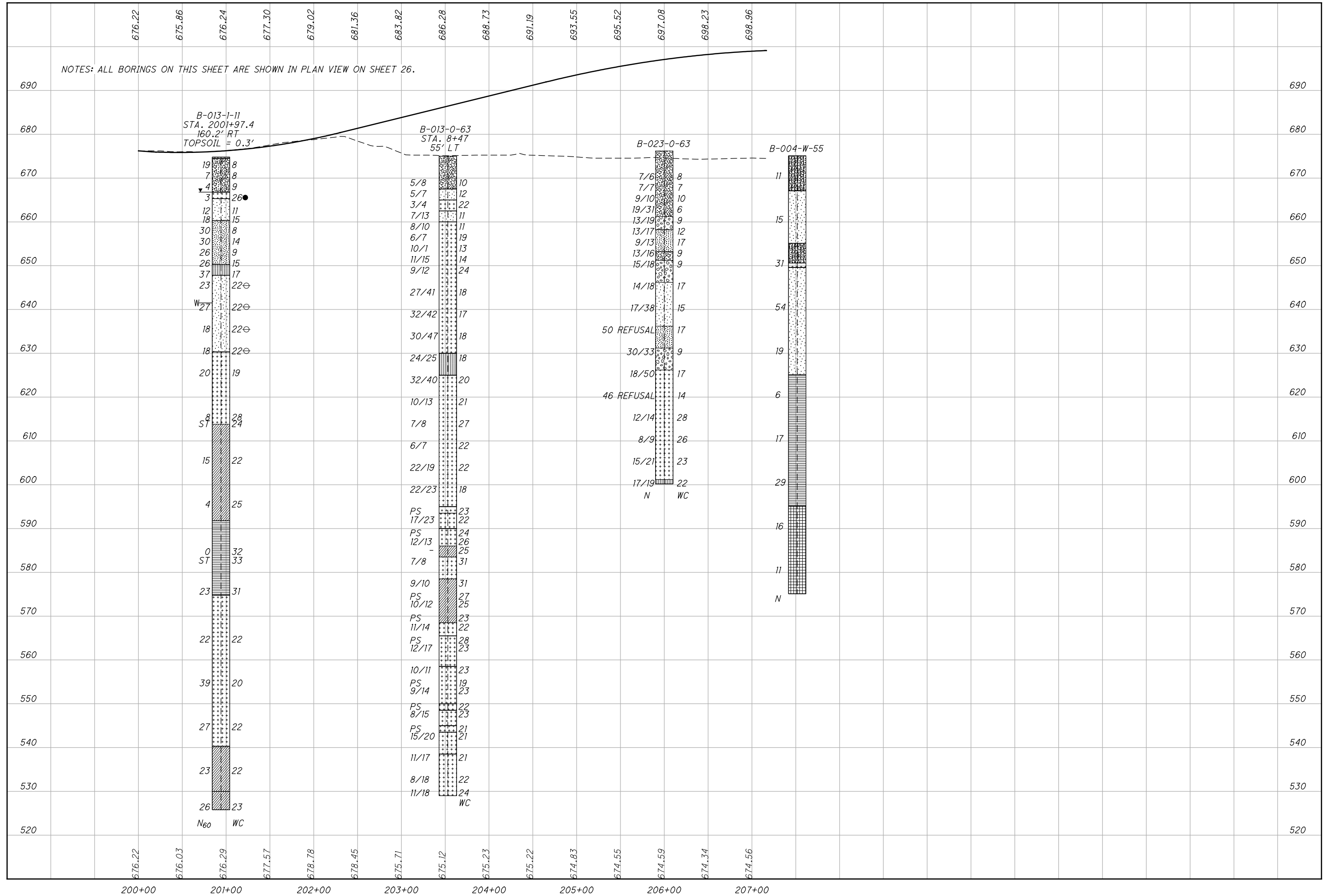


DRAWN  
CMH  
CHECKED  
JEP

**SOIL PROFILE**  
**STA. 2063+50 TO STA. 2067+00 IR 90 EB**

**CUY-90-14.90**





DRAWN: CMH  
CHECKED: JEP

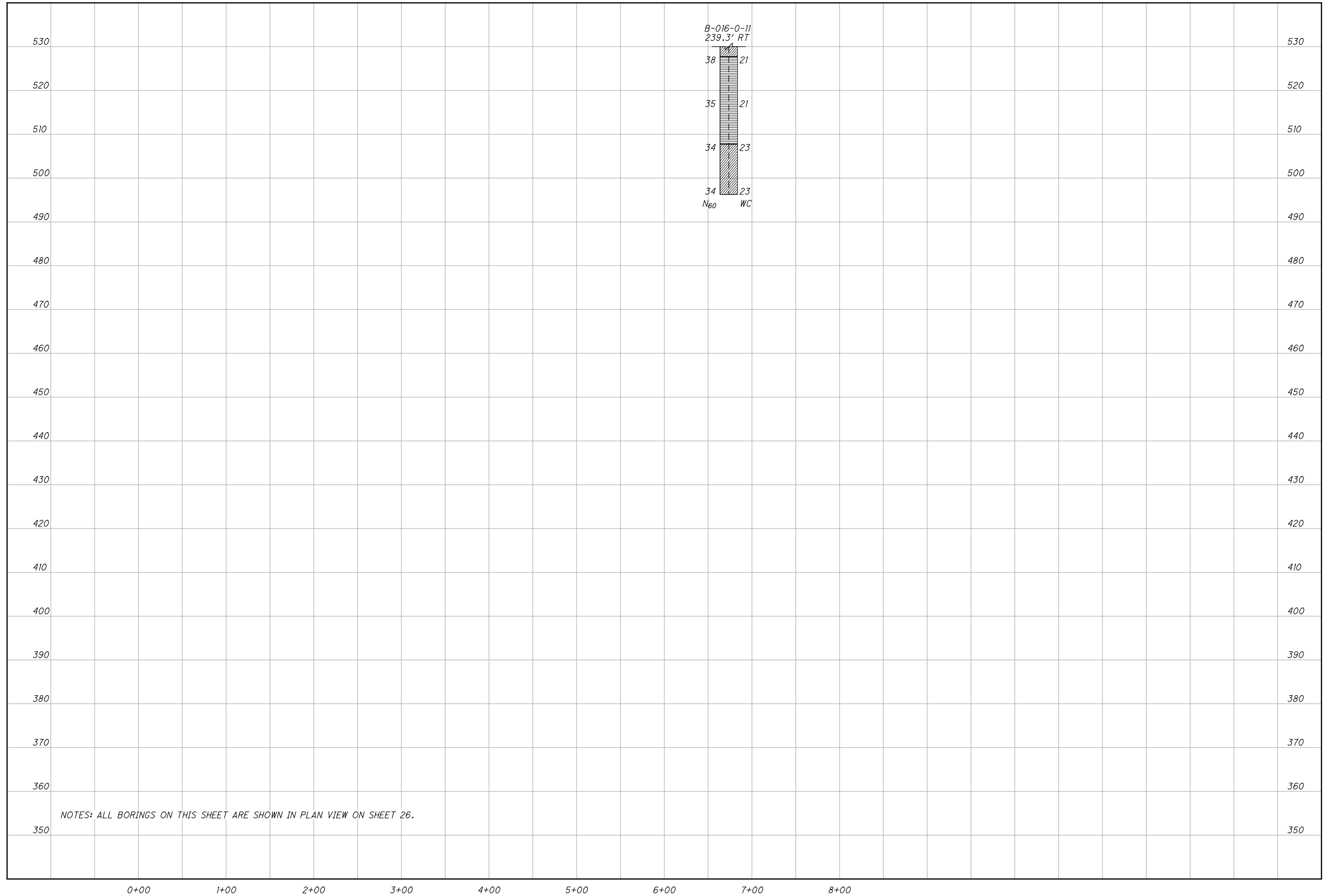
**SOIL PROFILE**  
**STA. 200+00 TO STA. 207+00 RAMP B2**

**CUY-90-14.90**





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DRAWN  
CMH  
CHECKED  
JEP

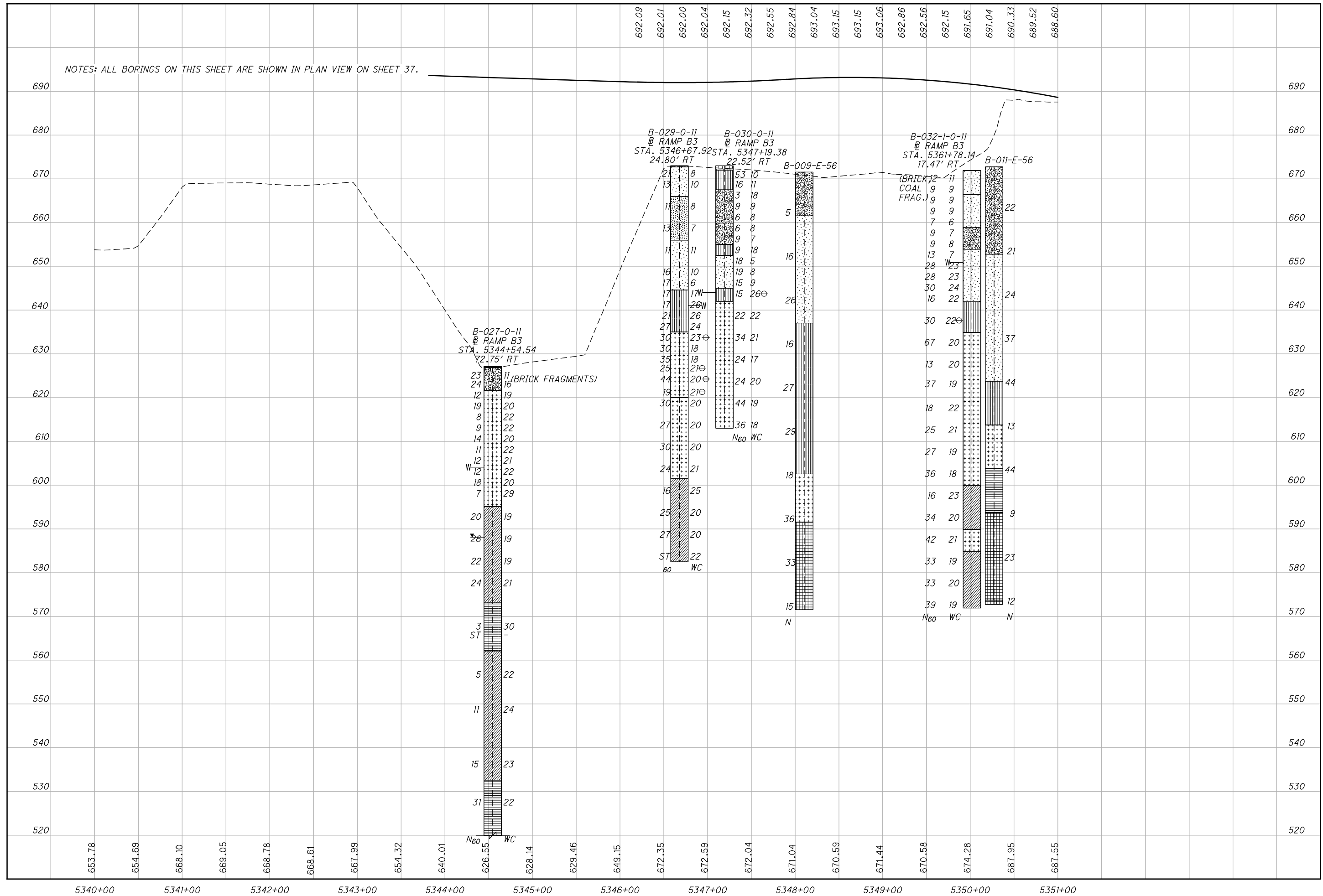
**SOIL PROFILE**  
**STA. 0+00 TO STA. 8+00 W. 14TH ST NB**

**CUY-90-14.90**





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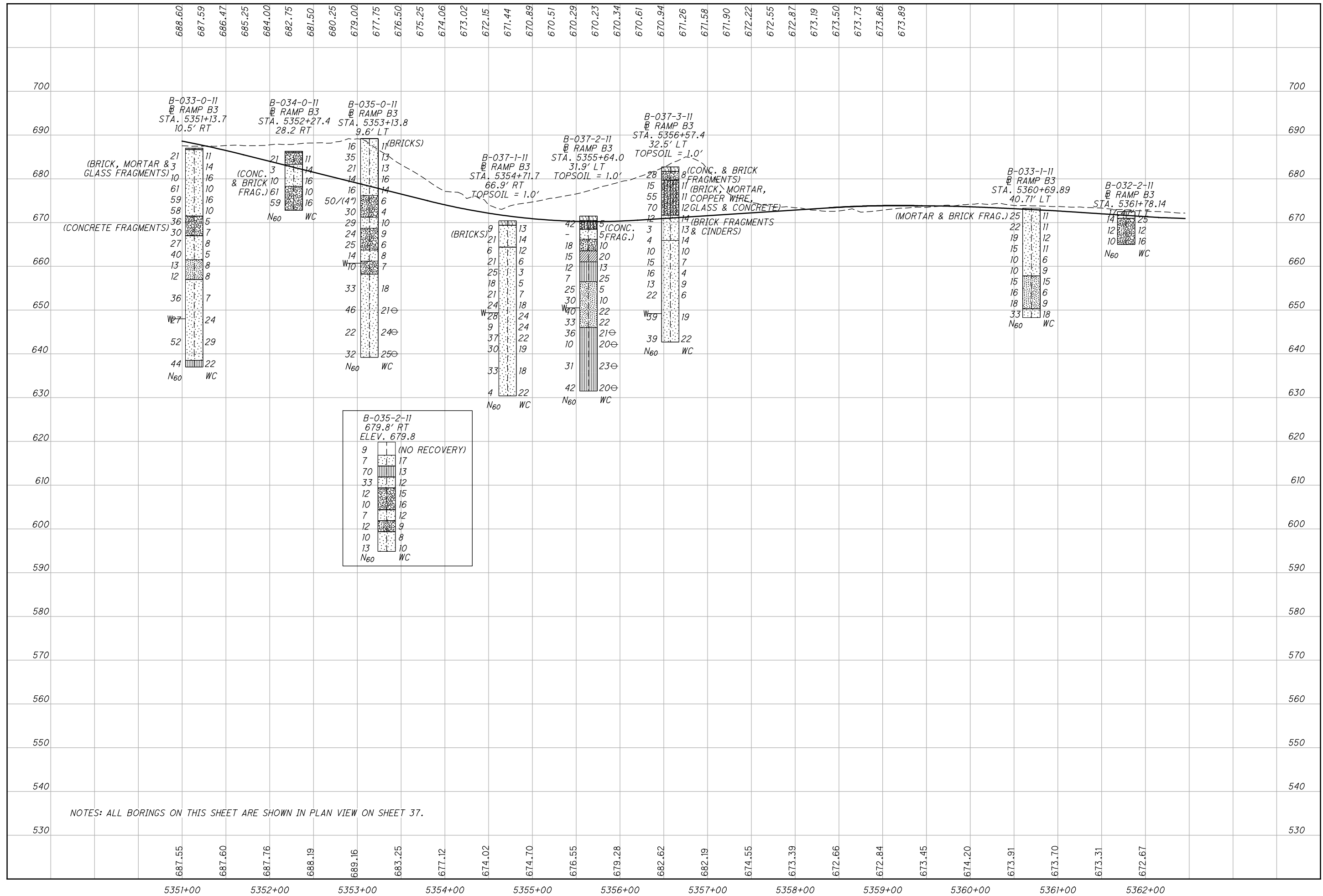


DRAWN CMH  
CHECKED JEP

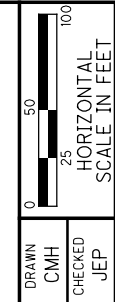
SOIL PROFILE  
STA. 5340+00 TO STA. 5351+00 RAMP B3

CUY-90-14.90

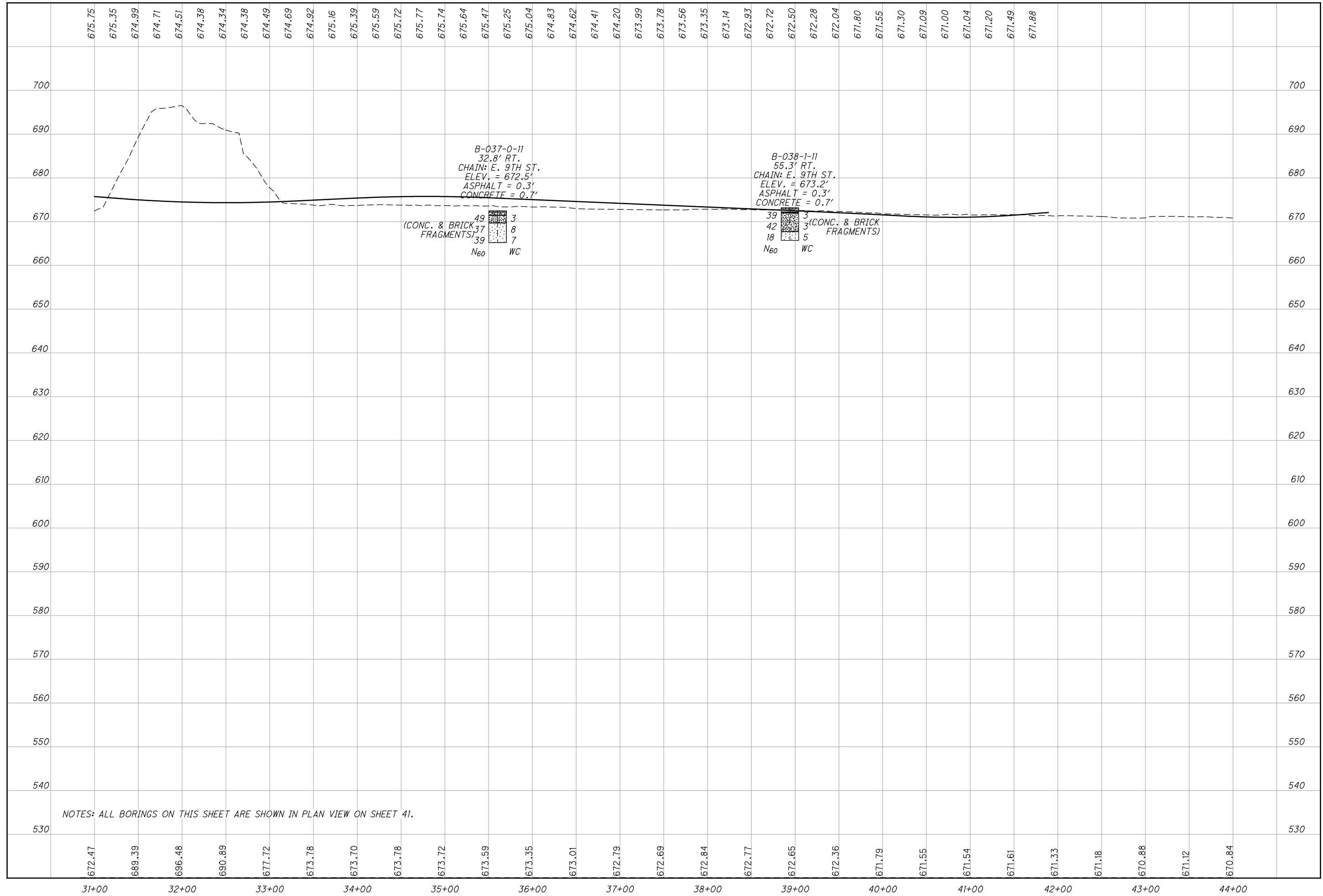




NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 37.



**SOIL PROFILE**  
**STA. 5351+00 TO STA. 5362+00 RAMP B3**

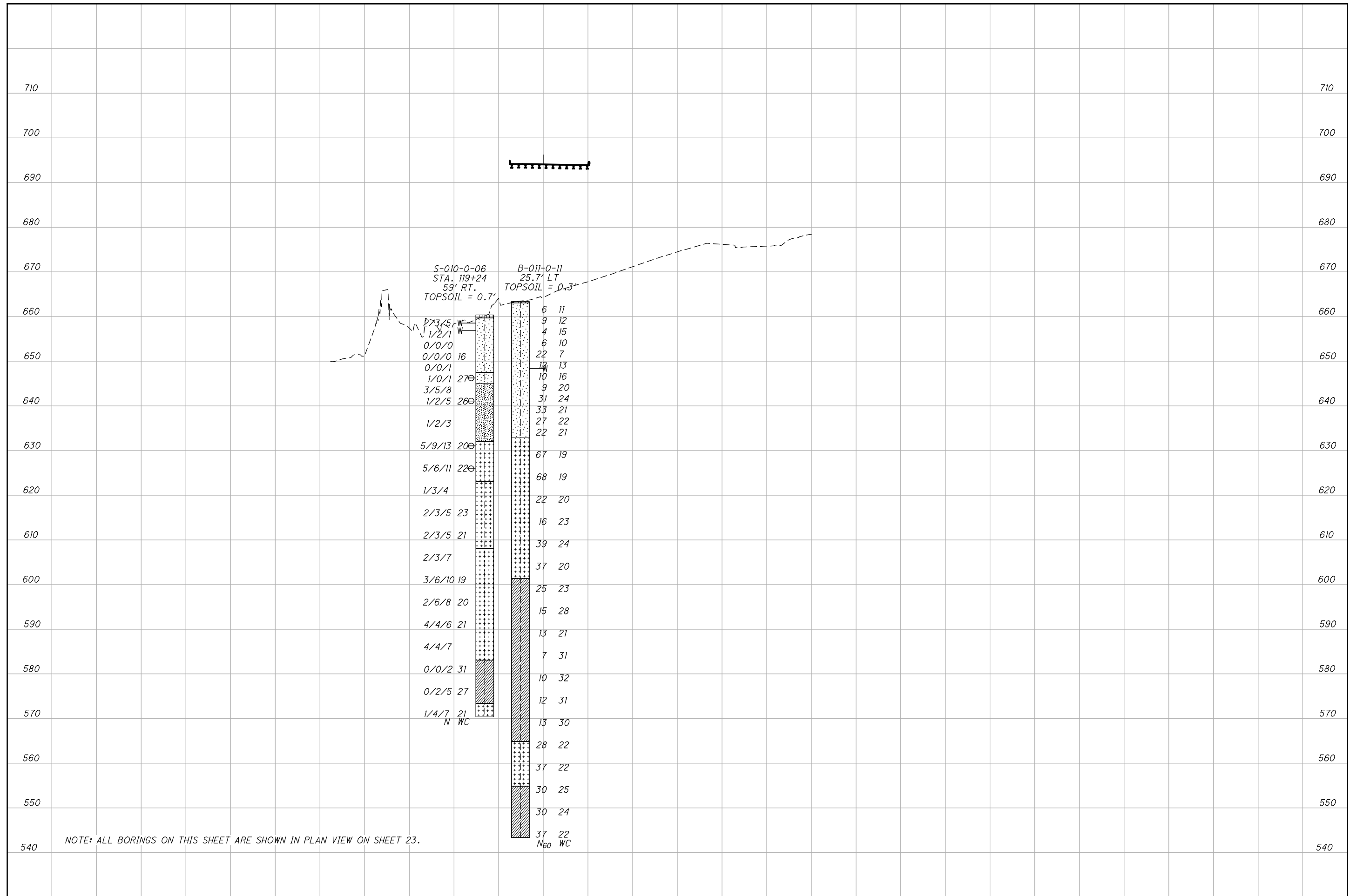


DRAWN CMH  
CHECKED JEP

**SOIL PROFILE**  
**STA. 31+00 TO STA. 44+00 E. 9TH ST.**

**CUY-90-14.90**





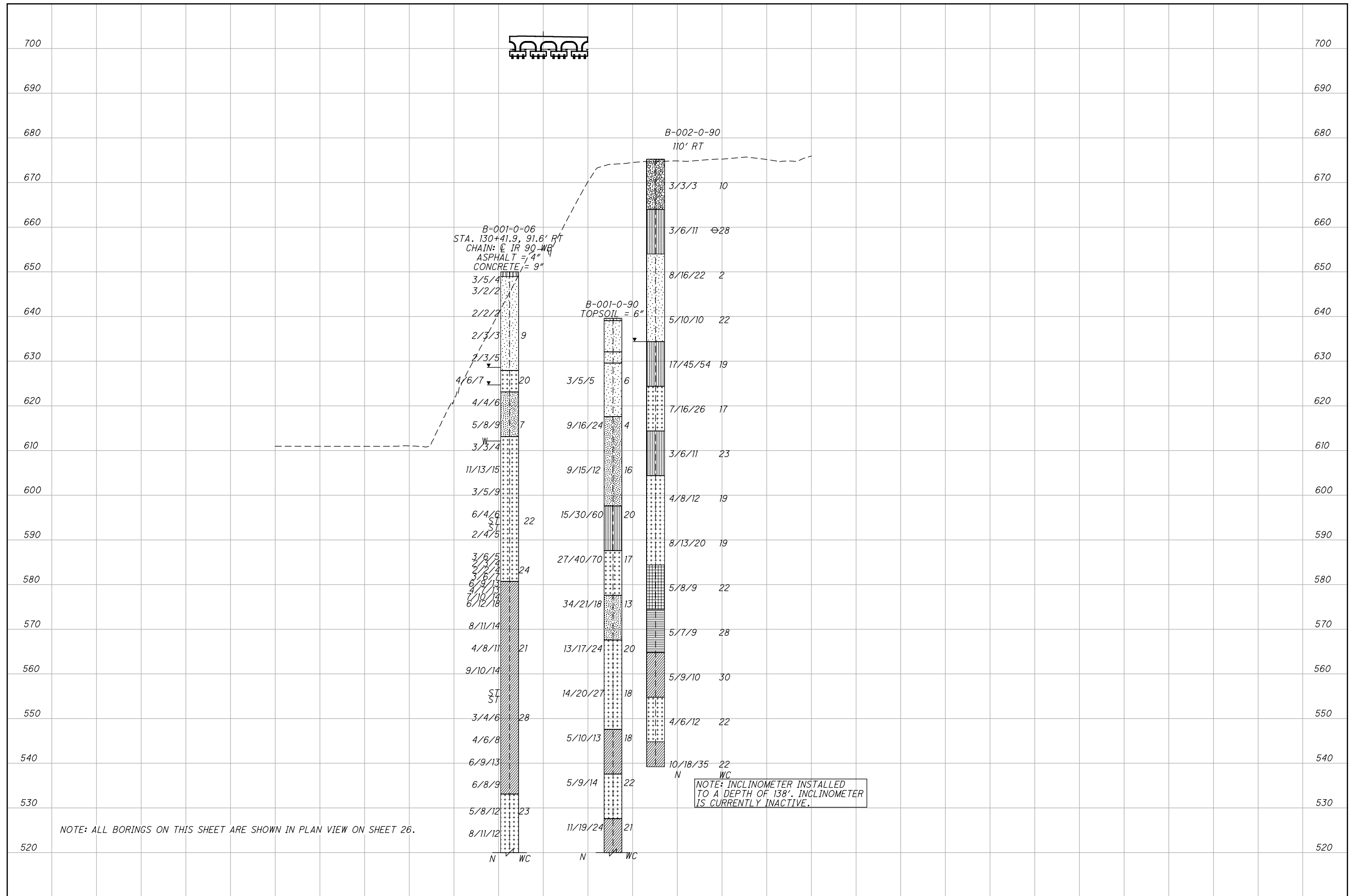
0 25 50 100  
 HORIZONTAL SCALE IN FEET  
 DRAWN CMH  
 CHECKED JEP

**SOIL PROFILE**  
**CROSS SECTION 1999+50 IR 90 EB**

**CUY-90-14.90**



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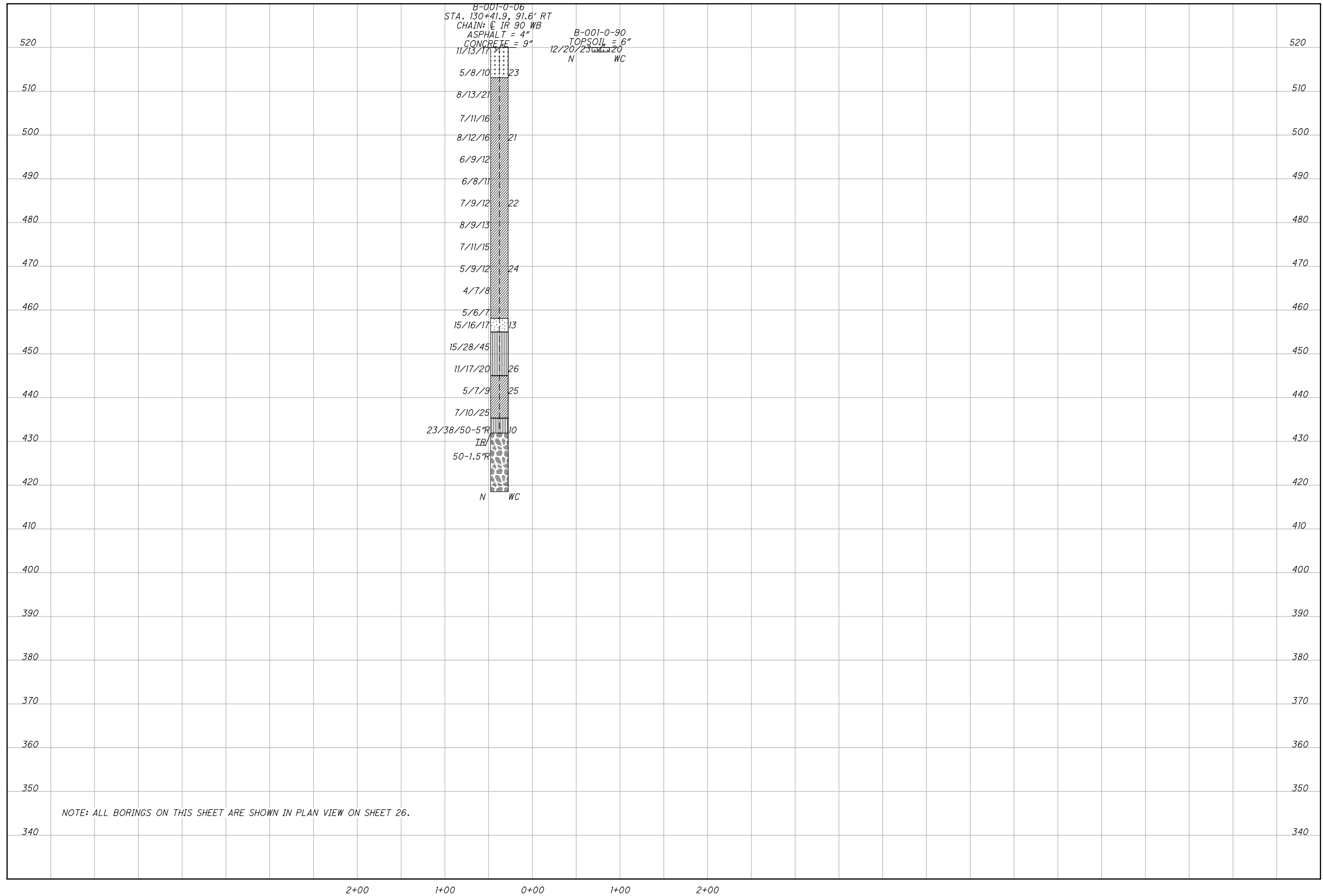
DRAWN: CMH  
 CHECKED: JEP

**SOIL PROFILE**  
**CROSS SECTION 2010+00 IR 90 EB**

**CUY-90-14.90**

52/67

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NOTE: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 26.



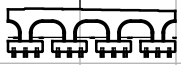
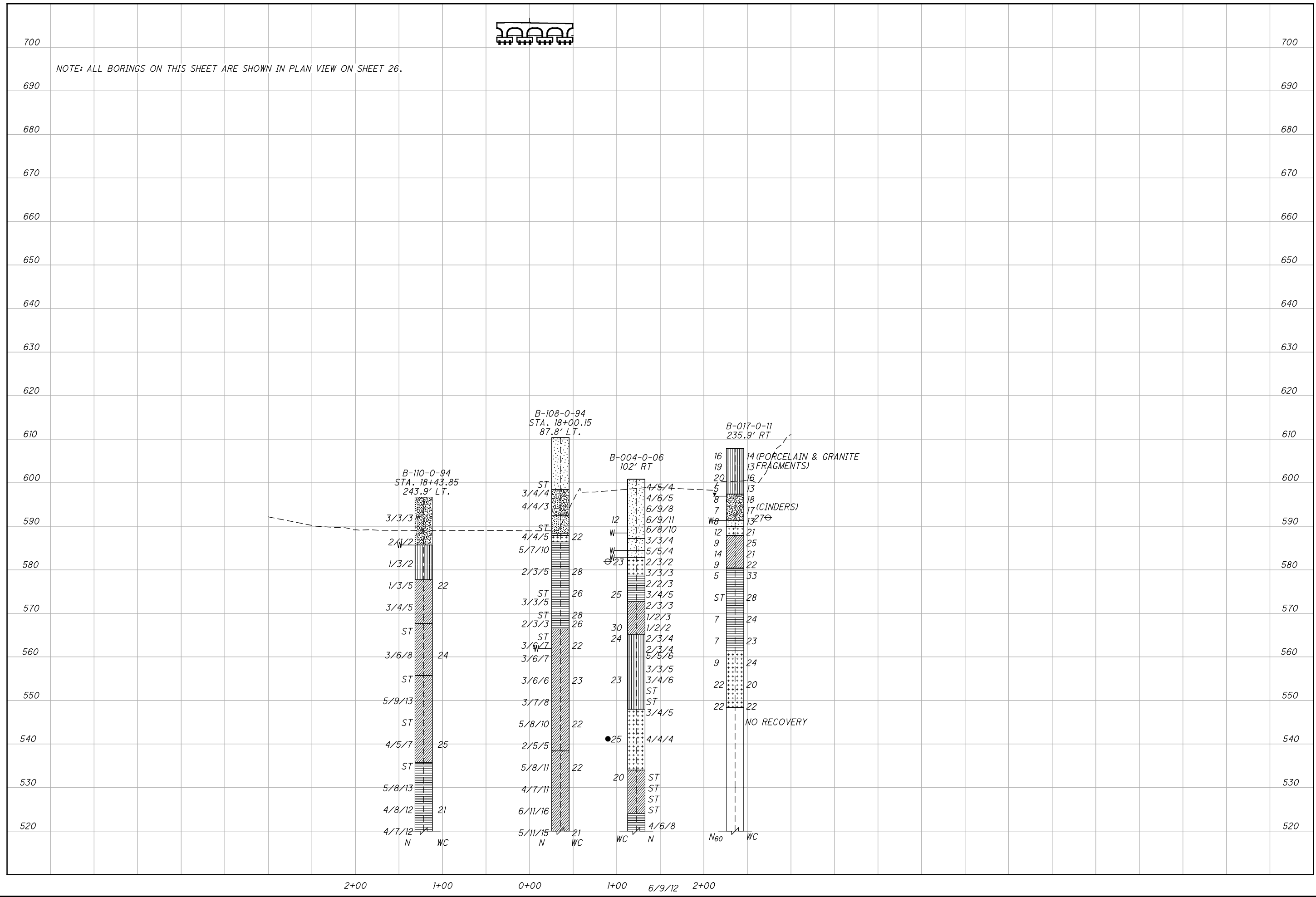
DRAWN	CMH
CHECKED	JEP

**SOIL PROFILE**  
**CROSS SECTION 2010+00 IR 90 EB**

**CUY-90-14.90**



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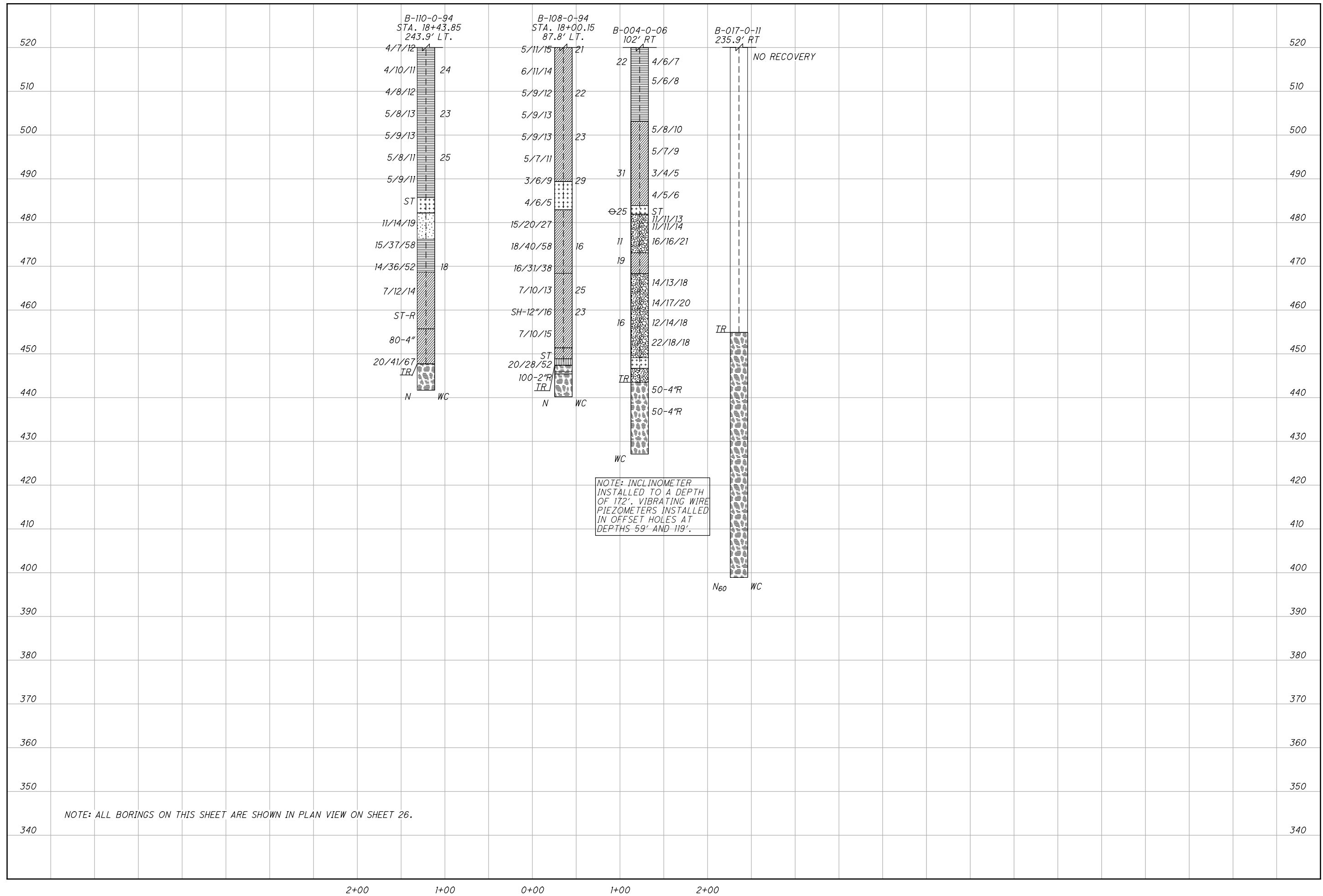


**SOIL PROFILE**  
**CROSS SECTION 2013+50 IR 90 EB**

**CUY-90-14.90**



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NOTE: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 26.



DRAWN CMH  
 CHECKED JEP

**SOIL PROFILE**  
**CROSS SECTION 2013+50 IR 90 EB**

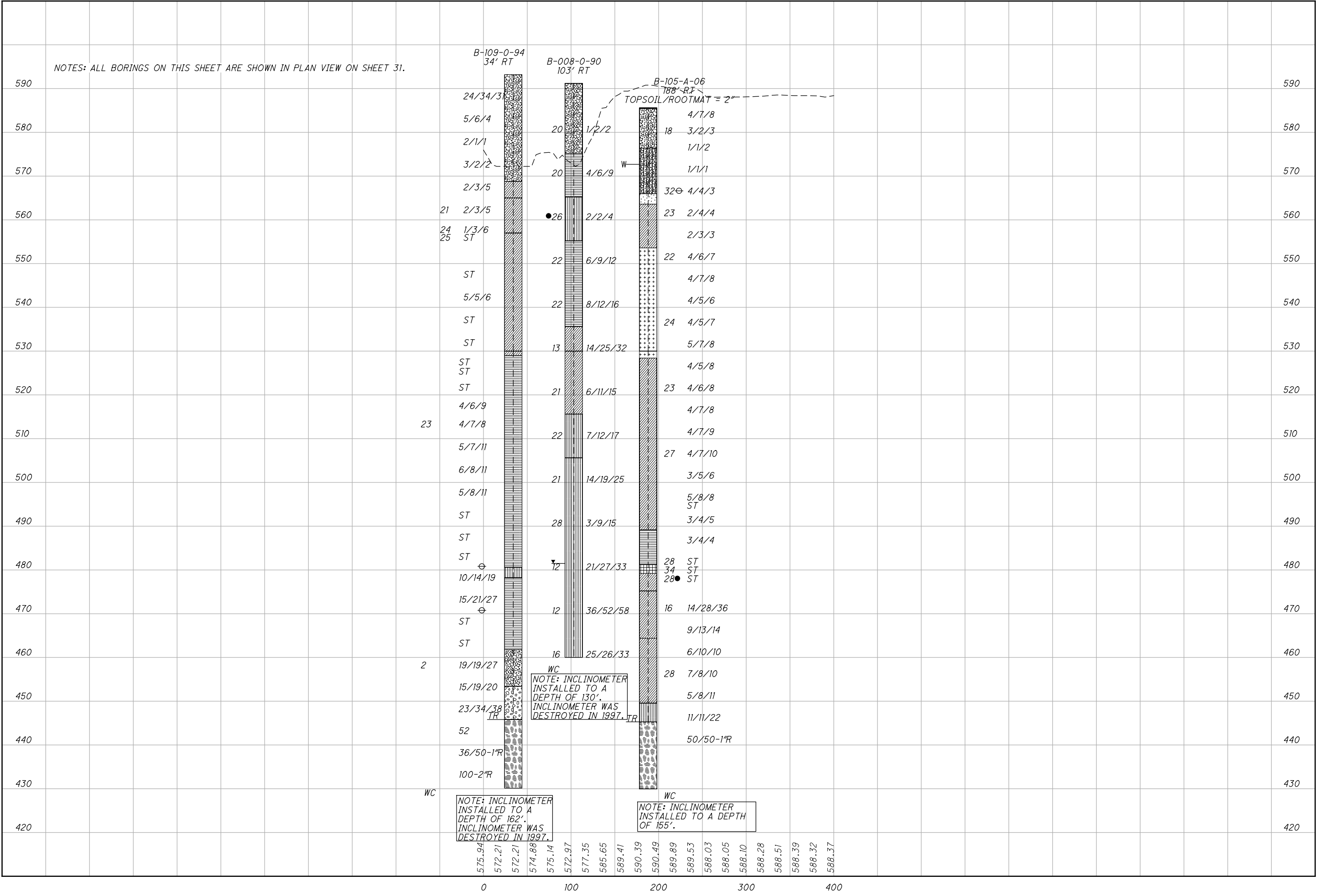
**CUY-90-14.90**





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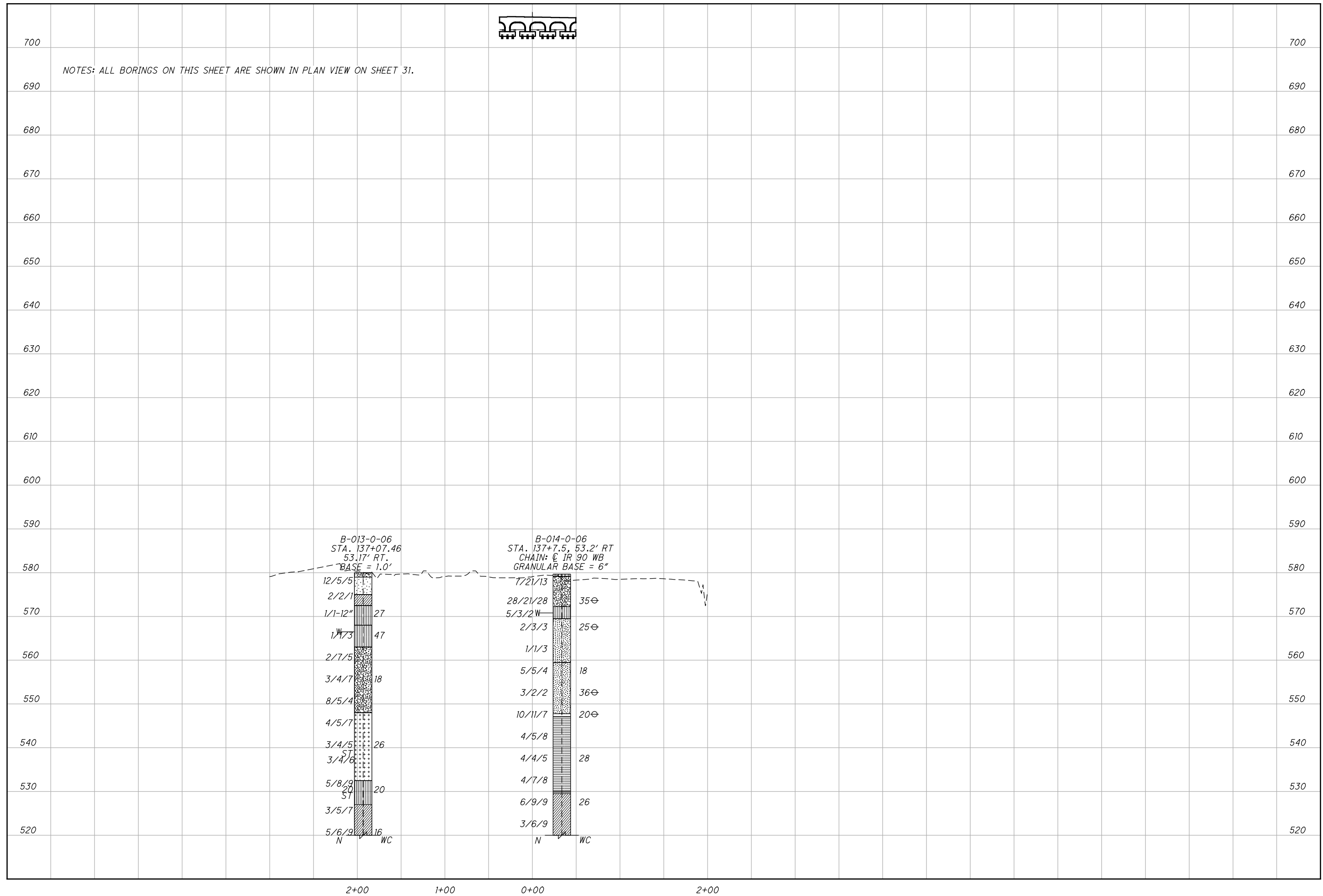
NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 31.



SOIL PROFILE  
CROSS SECTION 2014+00 RT IR 90

CUY-90-14.90

F:\CUIY\B2119\geotechnical\sheets\B2119\F027.dgn 7/20/2012 7:20:53 PM che

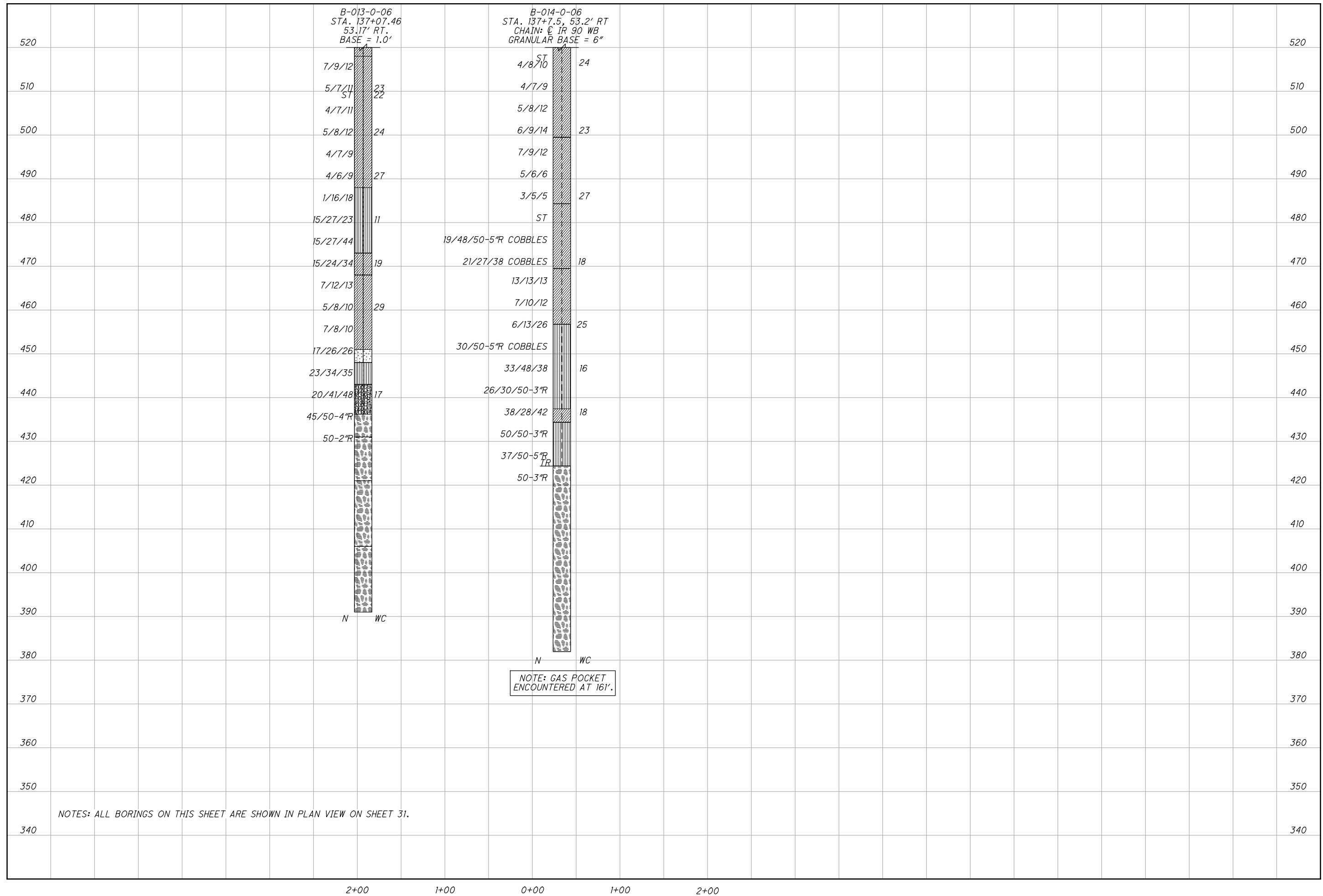


DRAWN: CMH  
 CHECKED: JEP

**SOIL PROFILE**  
**CROSS SECTION 2017+00 IR 90 EB**

**CUY-90-14.90**





NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 31.

NOTE: GAS POCKET ENCOUNTERED AT 161'.



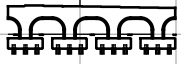
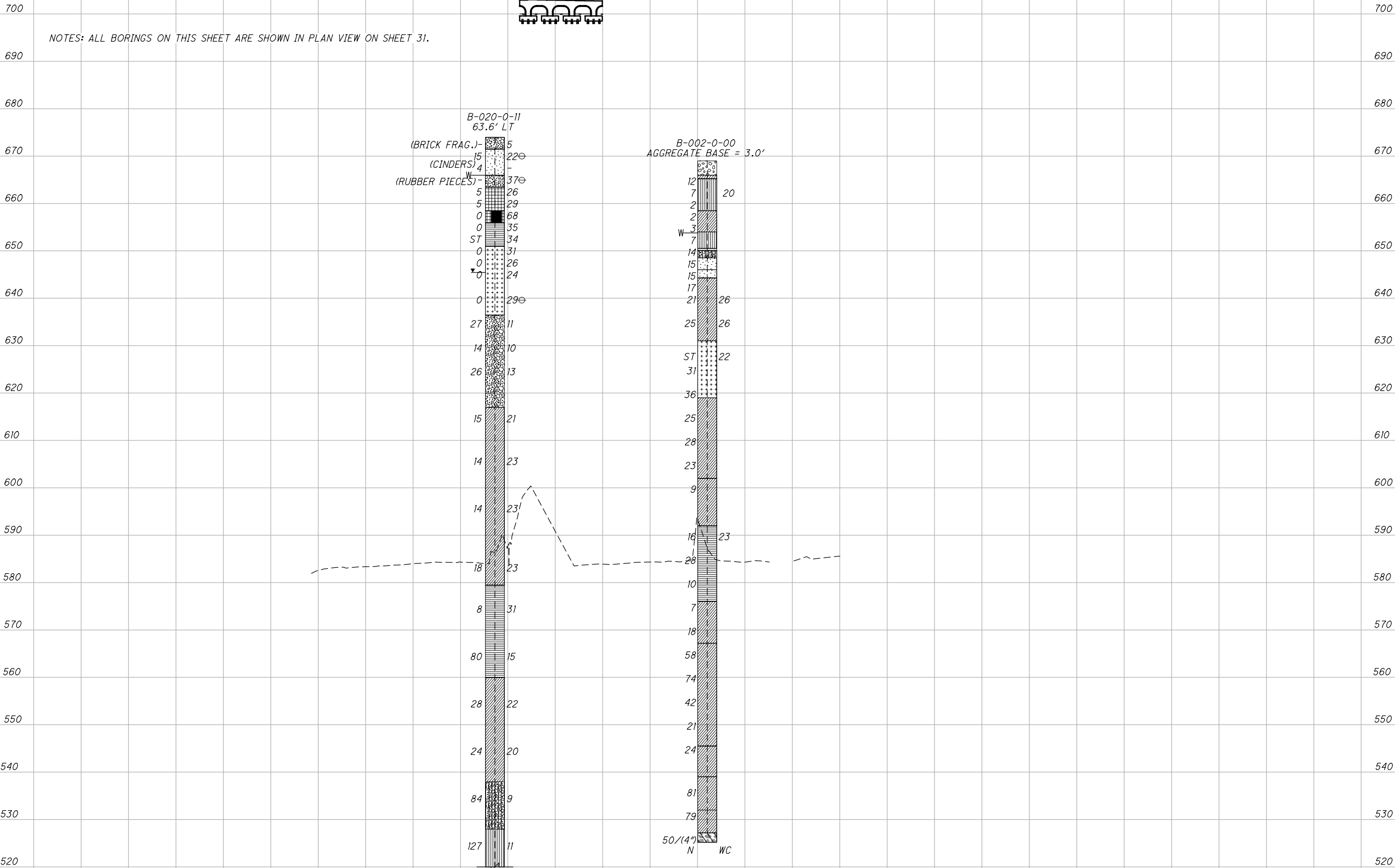
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
CROSS SECTION 2017+00 IR 90 EB

CUY-90-14.90



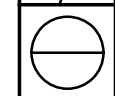
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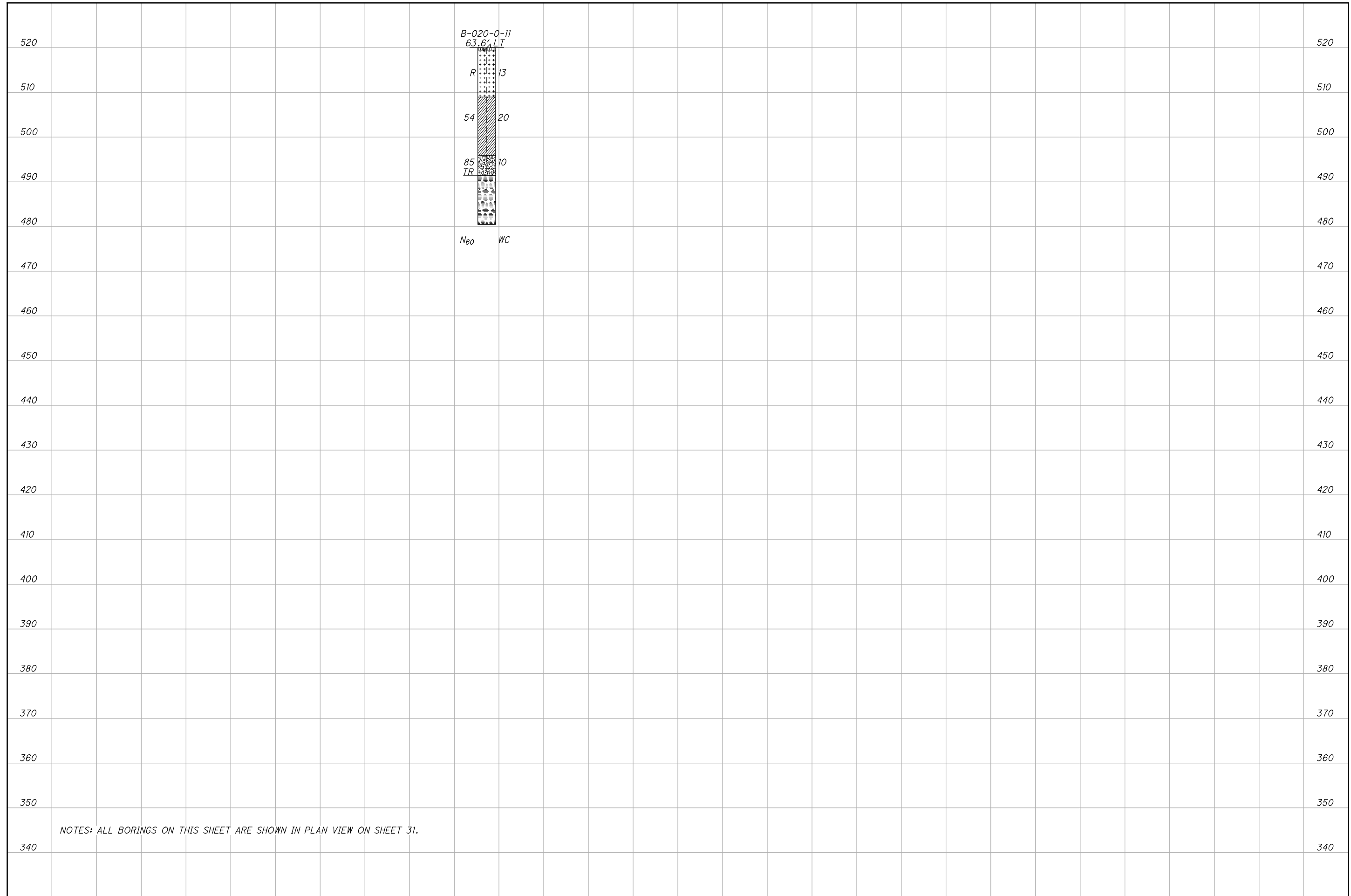
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
CROSS SECTION 2025+00 IR 90 EB

CUY-90-14.90



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NOTES: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 31.



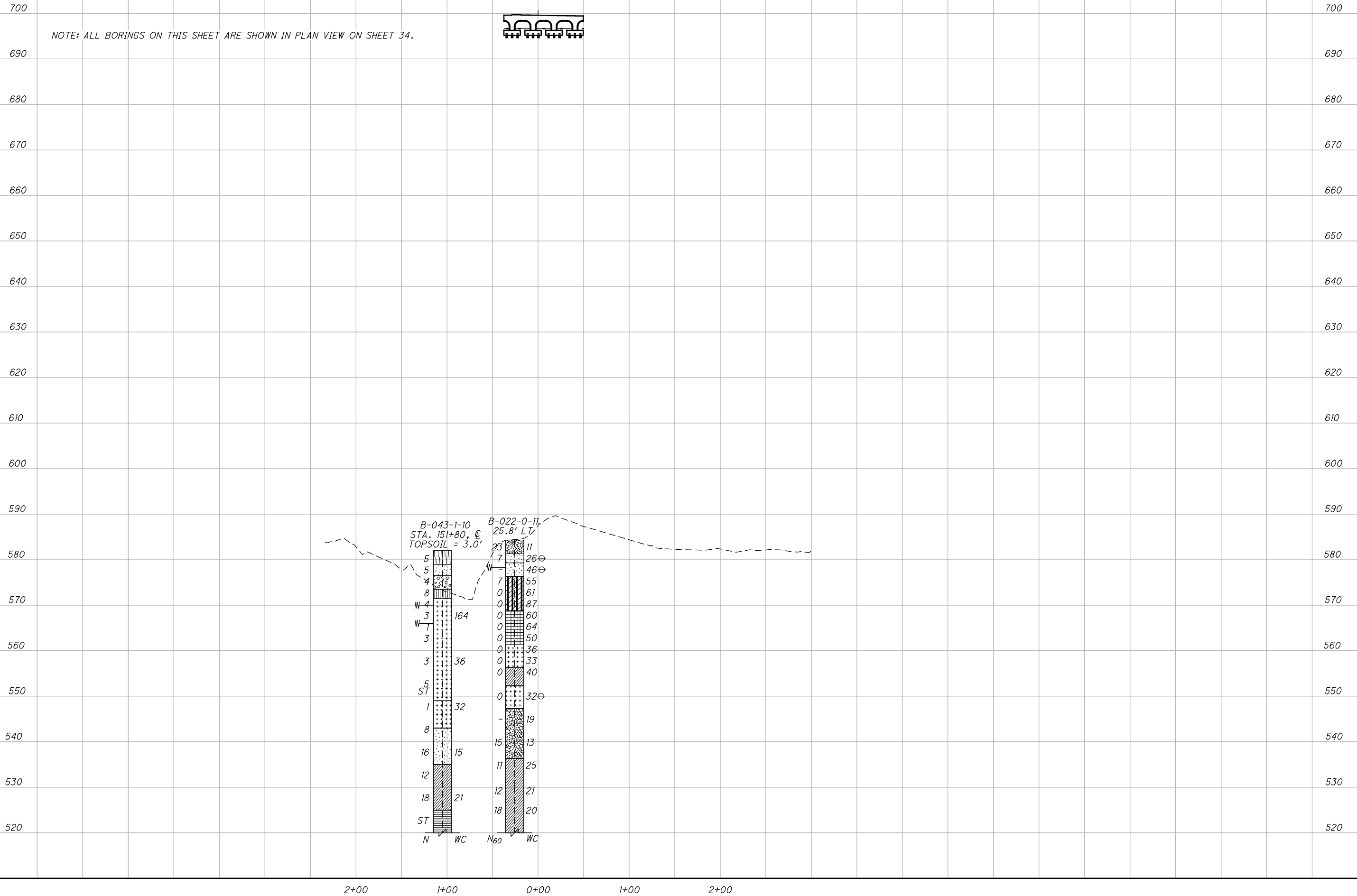
DRAWN	CMH
CHECKED	JEP

**SOIL PROFILE**  
**CROSS SECTION 2025+00 IR 90 EB**

**CUY-90-14.90**



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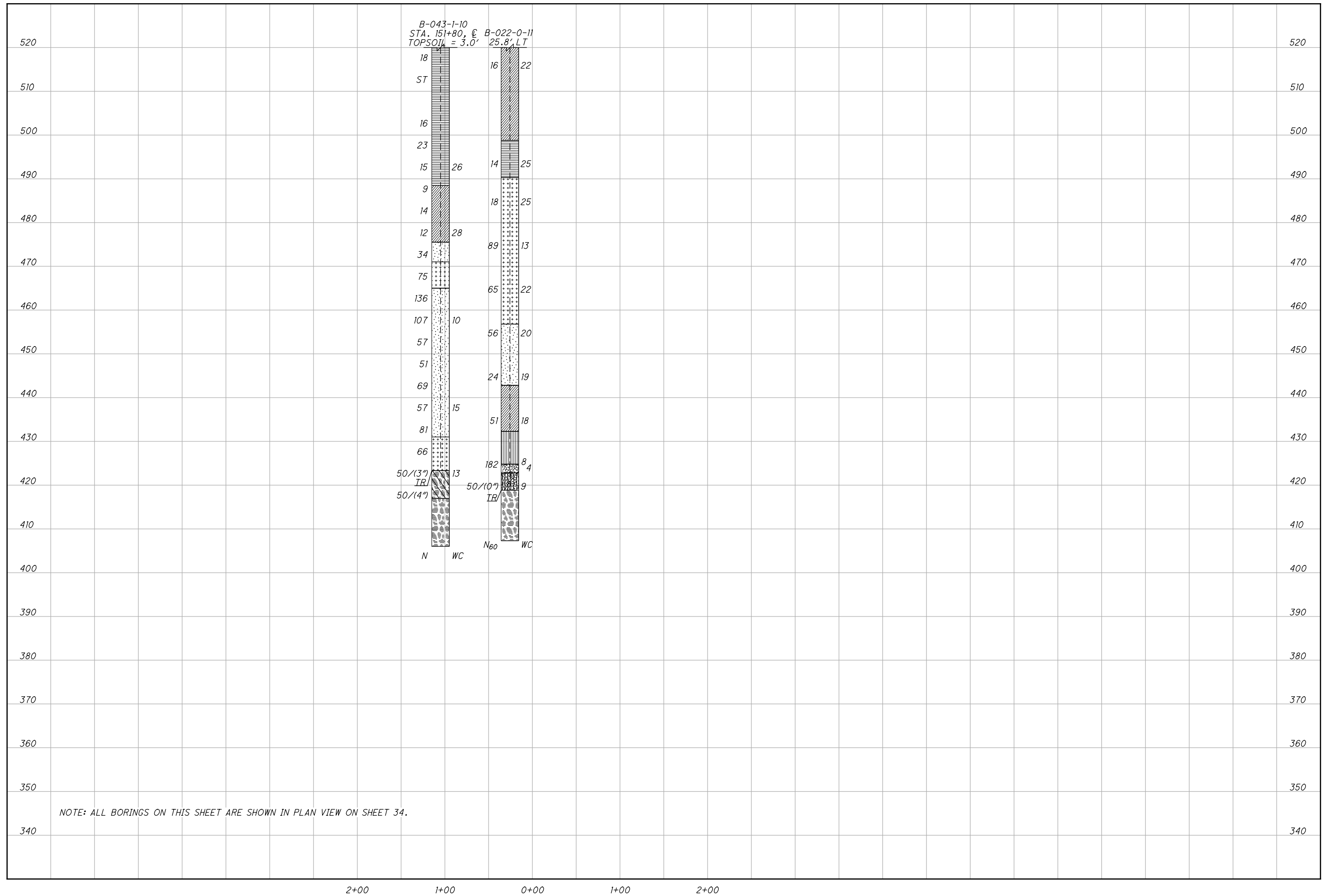
DRAWN: CMH  
CHECKED: JEP

SOIL PROFILE  
CROSS SECTION 2031+50 IR 90 EB

CUY-90-14.90



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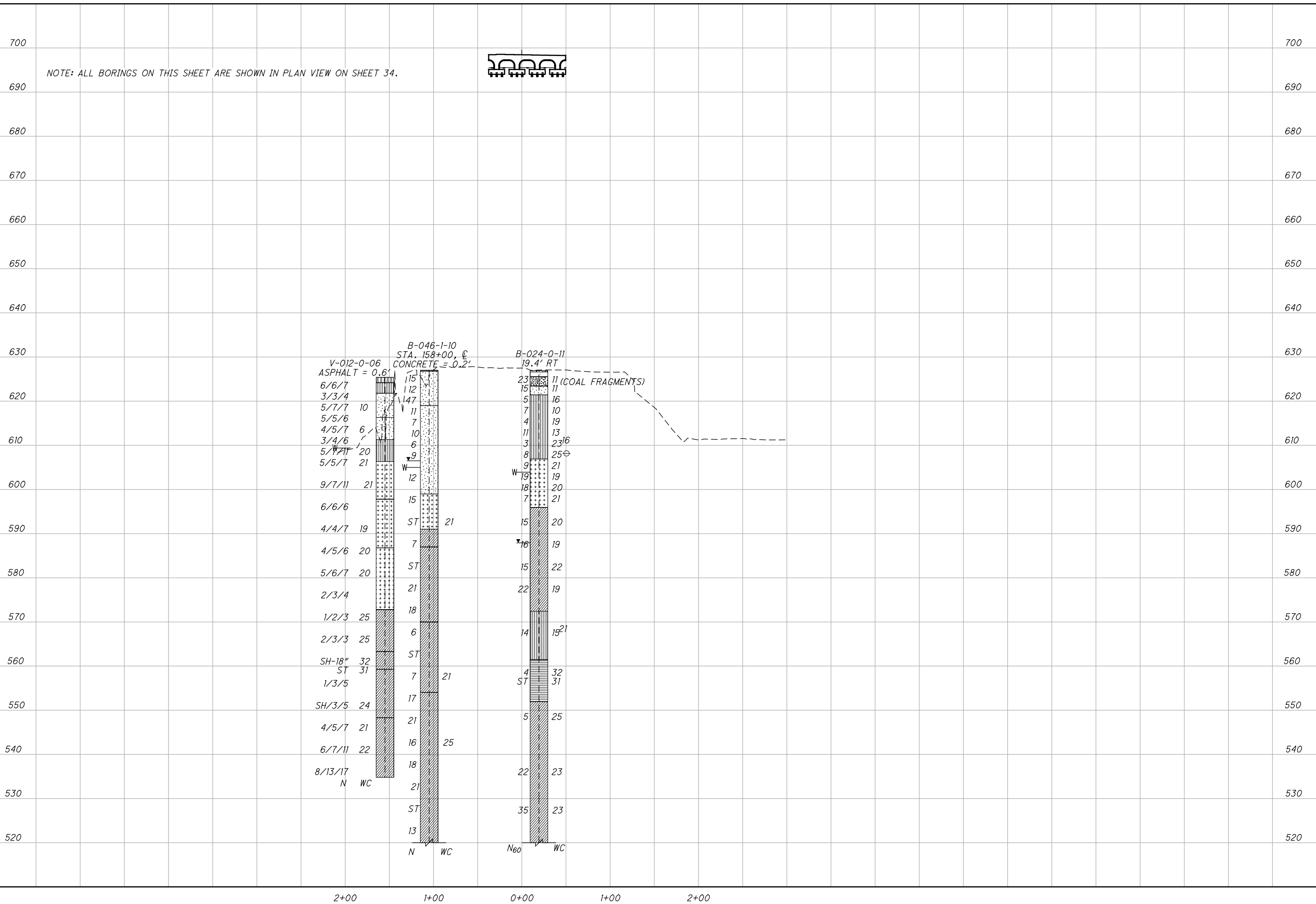
DRAWN: CMH  
 CHECKED: JEP

**SOIL PROFILE**  
**CROSS SECTION 2031+50 IR 90 EB**

**CUY-90-14.90**



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0 50 100  
25  
HORIZONTAL  
SCALE IN FEET

DRAWN  
CMH  
CHECKED  
JEP

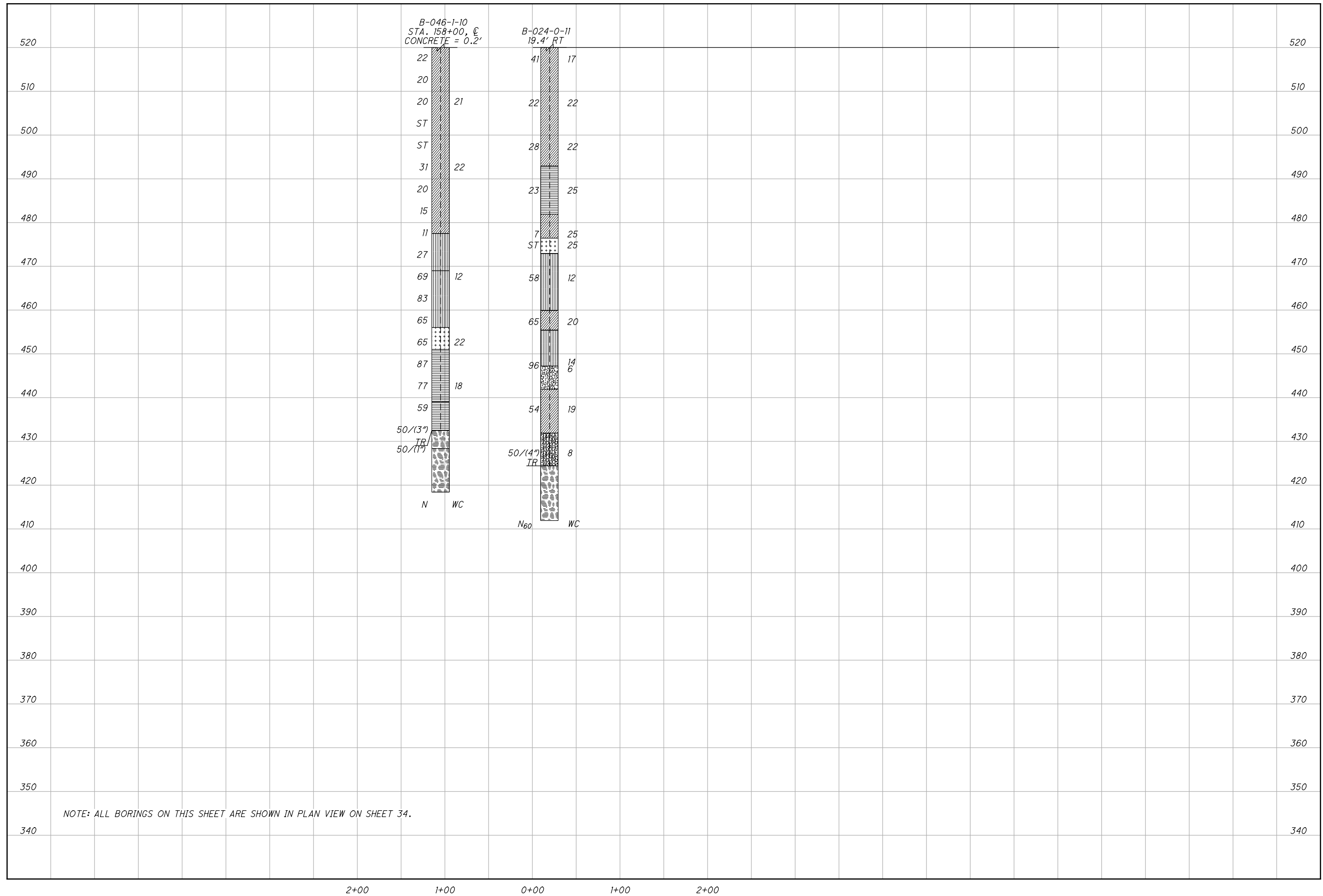
SOIL PROFILE  
CROSS SECTION 2037+50 IR 90 EB

CUY-90-14.90





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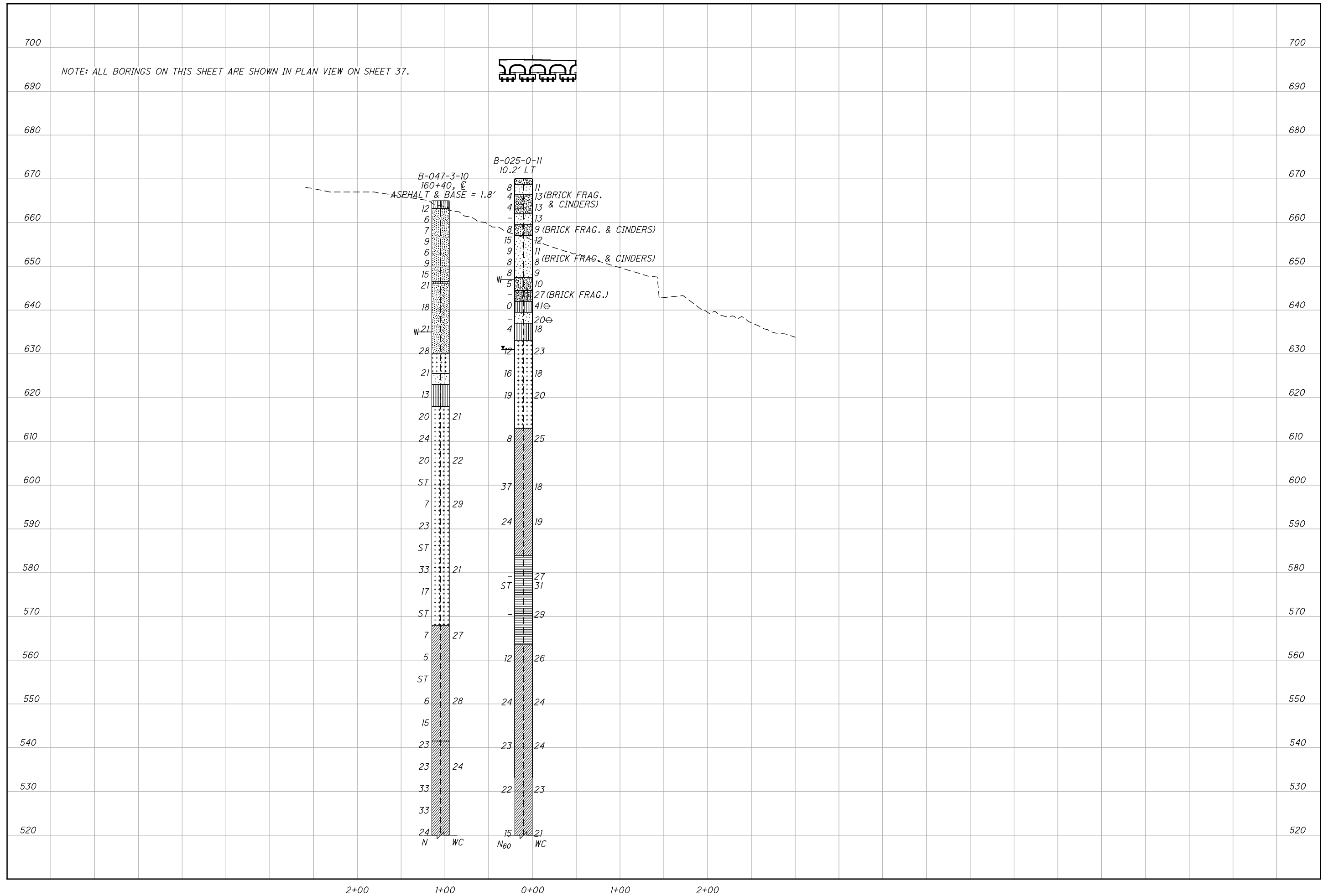
DRAWN: CMH  
CHECKED: JEP

**SOIL PROFILE**  
**CROSS SECTION 2037+50 IR 90 EB**

**CUY-90-14.90**



NOTE: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 34.



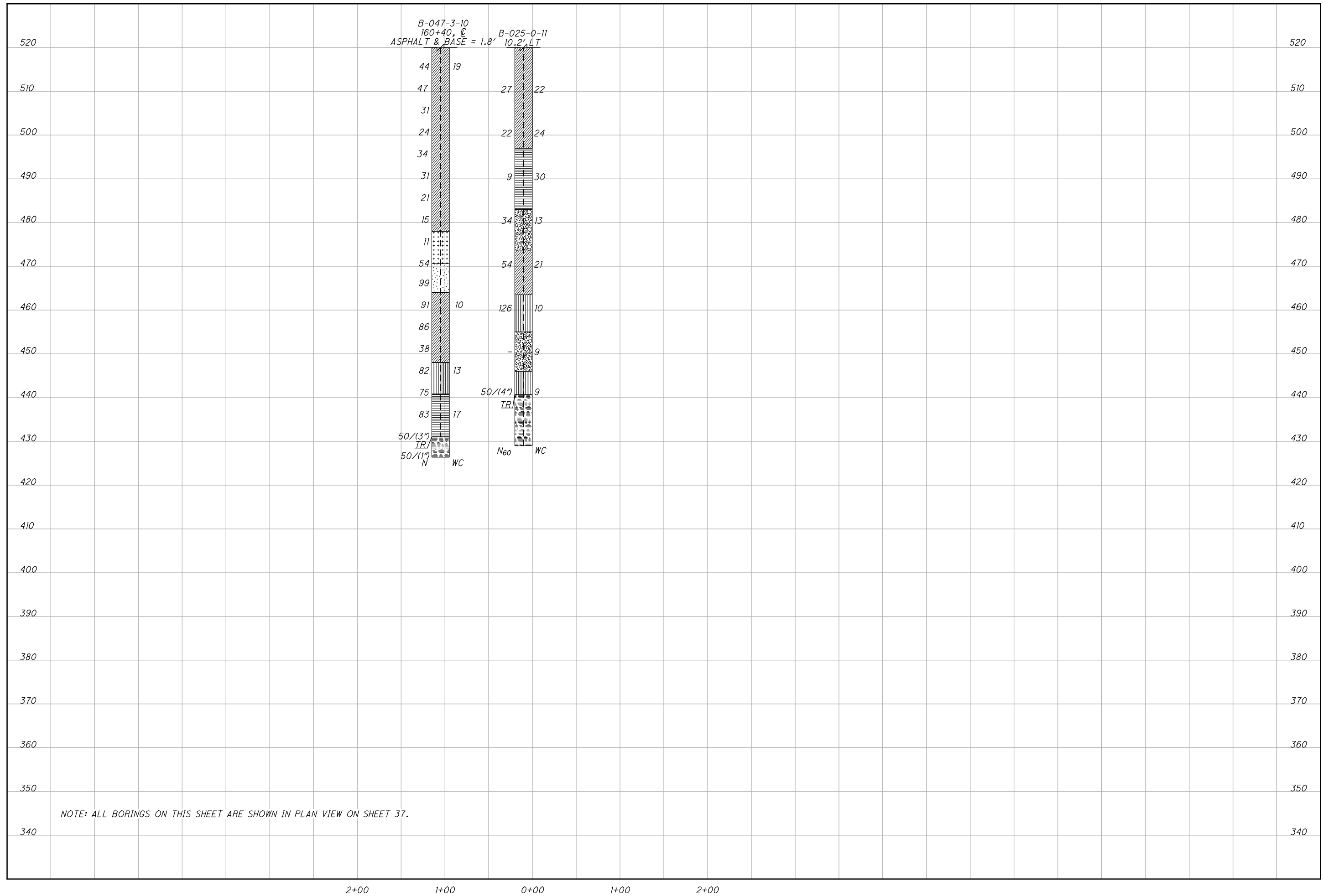
DRAWN CMH  
CHECKED JEP

SOIL PROFILE  
CROSS SECTION 2040+00 IR 90 EB

CUY-90-14.90



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NOTE: ALL BORINGS ON THIS SHEET ARE SHOWN IN PLAN VIEW ON SHEET 37.

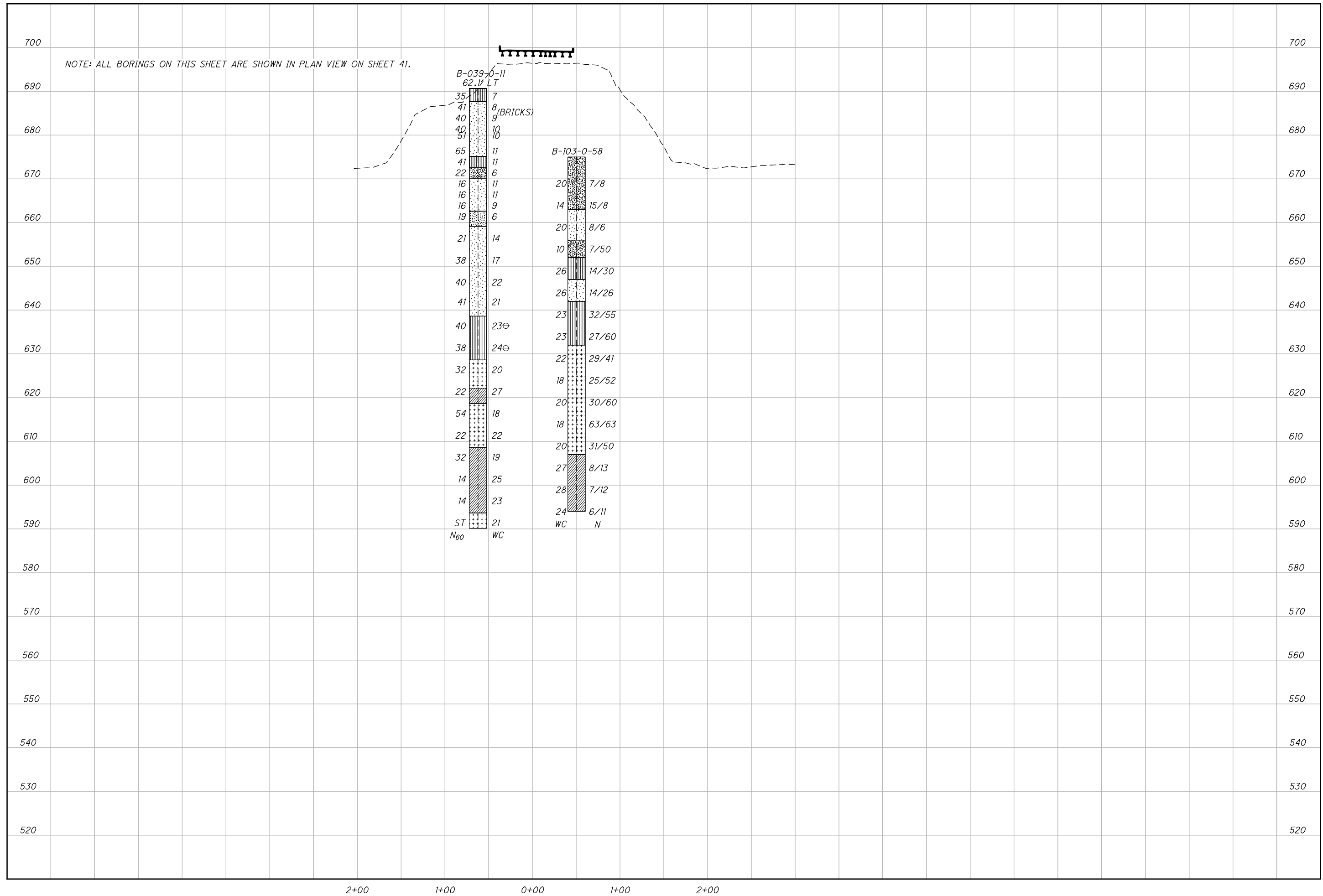


DRAWN	CMH
CHECKED	JEP

**SOIL PROFILE**  
**CROSS SECTION 2040+00 IR 90 EB**

**CUY-90-14.90**





DRAWN: CMH  
CHECKED: JEP

**SOIL PROFILE**  
**CROSS SECTION 2057+00 IR 90 EB**

**CUY-90-14.90**

