# **Ohio Bridge Inspection Summary Report**

2: District 12 16000 - CLEVELAND (CUY county)				
21: Major Maint A/B	01 - State Highway Agency			
225 Routine Main A/B 01 - State Highway Agency				
221 Inspection A/B	01 - State Highway Agency			
220: Inv. Location CUY				

# CUY-00176-1334 (1805436)

 5A: Inventory Route
 1
 00176

 7: Facility On
 SR 176 NB
 5

 6: Feature Ints
 IR-71NB (CUY-71-1791R)
 9: Location

 APPROX 1 MI S I-90
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	Condition	Str	ructure Type		
58: Deck 6 - Satisfactory Condition		43: Bridge Type 4 - Steel continuous			
58.01 Wearing Surface	7 - Good (1% distress)	02 - Stringer/Multi-beam or Girder			
58.02 Joint	4- Poor (heavy leaking, offset)	N- Not Applicable			
59: Superstructure	5 - Fair Condition	45: Spans Main / Approach 18 / 0			
59.01 Paint & PCS	7 - Good (1-5% corr.)	107: Deck Type	1 - Concrete Cast-in-Place		
60: Substructure	6 - Satisfactory Condition	408: Composite Deck	N - Non-composite Construction		
61: Channel	N	414A Joint Type 1	8 - Elastomeric Strip Seal		
61.01 Scour	N - Not Applicable	414B: Joint Type 2	N - None		
62: Culverts	N - Not Applicable	108A: Wearing Surface	1 - Monolithic Concrete		
			(concurrently placed with structu deck)		
67.01 GA	5		N- Not Applicable		
	Appraisal	422: WS Date			
36: Rail, Tr, Gd, Term Std	1 1 1 1	423: WS Thick (in)	1.2		
72: Approach Alignment	8 - Equal to present desirable criteria	482: Protective Coating	0 - Other Paint		
113: Scour Critical	N - Not over waterway	483: PCS Date	01/01/1991		
71: Waterway Adequacy	N - Not Applicable	453: Bearing Type 1	2 - Rockers & Bolsters		
		455: Bearing Type 2 N - None			
Geometric		528: Foundn: Abut Fwd 1 - Steel H Piles (Other size)			
48: Max Span Length (ft)	79.0		1 - Steel H Piles (Other Size)		
49: Structure Length (ft)	1073.0	536: Foundn: Pier 1	1 - Steel H Piles (Other size)		
52: Deck Width, Out-To-O		539: Foundn: Pier 2	0 - Other		
424: Deck Area (sf) 57942.0		Age and Service			
32: Appr Roadway Width (		27: Year Built/ 106 Reha			
51: Road Width, Curb-Curl		42A: Service On			
50A: Curb/SW Width: Left		42A. Service Under	1 - Highway 0 - Other		
50A: Curb/SW Width: Righ		28A: Lanes on	03		
34: Skew (deg)	0	28B: Lanes Under	00		
33: Bridge Median	0 - No median	19: Bypass Length	1		
54B: Min Vert Undercleara		29: ADT	75259		
336A: Min Vert Clrnce IR C		109: % Trucks (%)	3		
336B: Min V Clr IR Non-Ca		109. /0 HUCKS (/0)	3		
578: Culvert Length (ft)	0	Ins	pections		
	Load Posting		Months		
41: Op/Post/Closed	A - Open	90: Routine Insp.	12 05/08/2020		
70: Posting 5 - Equal to or above legal loads		92A: FCM Insp. N	24		
70.01: Date	-	92B: Dive Insp. N	0		
70.02: Sign Type		92C: Special Insp. N	0		
734: Percent Legal (%)	150	92D: UBIT Insp. Y	24 05/08/2020		
704: Analysis Date	07/01/2013	92E: Drone Insp.			
63: Analysis Method	6 - Load Factor (LF) rating reported by rating factor (RF) method using MS18	Inspector Miller, Jeff			

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rating factor (RF) method using MS18 loading.

#### **Bridge Inspection Report**

# **Element Inspection**

	Environment	Total Quantity	Units	Condition State 1	Condition State 2	Condition State 3	Condition State 4
12 - Reinforced Concrete Deck	3 - Mod.	67032	sq. ft.	62819	1171	2850	192
805 - Wearing Surface - Monolithic Concrete		68951	sq. ft.	60329	8317	305	0
107 - Steel Open Girder/Beam	3 - Mod.	11371	ft.	10806	565	0	0
515 - Steel Protective Coating		127657	sq. ft.	117537	9036	1084	0
205 - Reinforced Concrete Column	3 - Mod.	47	each	41	2	4	0
210 - Reinforced Concrete Pier Wall	3 - Mod.	157	ft.	70	87	0	0
215 - Reinforced Concrete Abutment	3 - Mod.	126	ft.	71	21	34	0
234 - Reinforced Concrete Pier Cap	3 - Mod.	771	ft.	507	202	62	0
300 - Strip Seal Expansion Joint	3 - Mod.	267	ft.	0	199	45	23
311 - Movable Bearing	3 - Mod.	160	each	86	57	17	0
321 - Reinforced Concrete Approach Slab	3 - Mod.	2259	sq. ft.	2024	24	199	12
331 - Reinforced Concrete Bridge Railing	3 - Mod.	3838	ft.	2583	729	526	0
815 - Drainage	3 - Mod.	18	each	6	4	6	2
820 - Steel Seated-Hinge Assembly	3 - Mod.	21	each	1	19	1	0
830 - Abutment Backwall	3 - Mod.	126	ft.	106	20	0	0

ODOT District:	12	(	CUY-00176	-1334 _(1805436)	Date Built: 07/01/1968
Major Maint:	01 - State Highway Agency	Facility Carried:	SR 176 NB	Traffic On: 1 - Highway	Rehab Date:
Routine Maint:	01 - State Highway Agency	Feature Inters:	IR-71NB (CUY-71-1	791R) Traffic Under: 0 - Other	Insp. 01 - State Highway Agency Resp A:
FIPS Code:	16000 - CLEVELAND (CUY county	)	Location: CUY	APPROX 1 MI S I-90	Insp
	Inspector Mil	ler,Jeff	Inspection Date	05/08/2020 12:00:00 Reviewer Lawler,Matthew	Resp B:

# Inspector Comments - Deck and Approach

#### <u>Deck</u>

# Floor/Slab (SF)

The floor exhibits widespread transverse cracking with and without efflorescence. Full Width x 2' L spalling along joints headers were noted over Pier 13. Other areas of spalling appears were noted between beams. In Unit 4BE, widespread areas of poor consolidation with and without efflorescence are common. Some areas have exposed reinforcement chairs, others have exposed reinforcement. In span 21BE, the floor exhibits a 16'x12' deep spall with 27 exposed and deteriorated rebar located south of Pier 21BE. In span 27BE, the concrete floor exhibits is a large 20'x8' damp area.

## Edge of Floor/Slab (LF)

The edge of the floor typically exhibits spalling at the expansion joints. Poor consolidation noted along the floor also expands out towards the edge the floor as well.

#### **Bridge Wearing Surface (SF)**

The wearing surface is in overall good condition. A few small potholes were noted at random locations. In span 19BW, there are multiple areas of abrasion on the wearing surface. In span 24BE, there is a 3'x8' partial patch with map cracks and settlement. In span 29BE, there is a 3'x3'x2.5" deep pothole.

## **Expansion Joint (LF)**

At all of the expansion joints, the metal is rusting and the joints are completely filled for most of the joint on the shoulders. At Expansion Joint 1B, the joint seal is broken and coming out of the joint on the left shoulder. At Expansion Joint 2BW, the seal is coming out of the joint on both shoulders and cars driving in the right lane make a sound when driving over the joint. At Expansion Joint 2BE, the joint seal is broken and coming out of the joint on the left side and the metal is sticking above the wearing surface on the left side. At Expansion Joint 3BE, the entire joint seal is wavy.

## **Bridge Railing (LF)**

Longitudinal, vertical, and areas of map cracking are common throughout the entire length of the bridge parapets. Discoloration along the cracks is evidence of water infiltration into the concrete parapet. At some of the joints, spalling at the bottom portions of the parapet were noted.

## Deck Drainage (EA)

Most scupper grates on the bridge are 25-75% filled with dirt and debris, but the downspouts are visible and are not clogged. There are a total of 5 clogged scuppers with grates that are completely filled with dirt and debris and are located on the right side of spans 8, 14, and 16. Multiple scupper downspouts exhibit minor section loss and are located on the right side of spans 14 and 25BE and the left side of span 27BE. There are scuppers with downspouts missing on the left side of span 19BW and the right side of span 25BE.

## Signs (EA)

A small director sign is mounted on the east side of the east parapet on the north side of Pier 12. One exit sign is mounted within the divide of the West 14th Street exit. In Unit 3BE, there are roundabout warning signs mounted on the parapets. In Unit 4BE, there are yield warning signs mounted on the parapets. The

existing overhead signs are mounted to pier caps supporting the overhead bridge CUY-71-1791.

## Sign Supports (EA)

No deficiencies were noted along the sign mounts.

#### **Utilities (LF)**

The conduit running along the bridge runs up the columns for I-71 (CUY-71-1791) to provide lighting along Jennings Highway. Multiple lights were not working during the inspection. Due to spalling along the columns for I-71 (CUY-71-1791) the conduit has become detached.

#### Approach

#### Approach Wearing Surface (EA)

The north approach for ramp BE exhibits longitudinal cracking along the pavement seams. Bituminous patch work along the steel joint header were also noted. Along the right shoulder, a 5' W x 3' L area of dense map cracking is evidence of minor settlement. The north approach for BW appears to be fairly new bituminous with minor cracks and minor debris accumulations along the shoulders. At the joint header, a 1' W x 1' L x 4" D pothole exists in the right wheel path of the right lane. Also, a 2' L x 2' W patch exists in the left wheel path. In the left lane, a 4' L x 2' W patch exists in the left wheel path, just north of the joint. The south approach wearing surface has a 3' W x 1' L depressed area along the joint header.

#### Approach Slab (SF)

The approach slabs are covered in a bituminous wearing surface and therefore not visible. The deterioration in the wearing surface is assumed to be reflected into the concrete approach slabs. The north approach slab BW has map cracks along the entire expansion joint, a large patch, and a wide crack on the left shoulder. The north approach slab BE has wide map cracks on the right shoulder and the middle lane line, minor map cracks in the right lane, and a 2'x6'x4" deep pothole near the left edge line.

#### Approach Relief Joint (LF)

The approach relief joints are covered in a bituminous wearing surface and therefore not visible. Any deterioration in the wearing surface is assumed to be reflected into the approach relief joints. The north approach BE relief joint has minor potholes in the left lane.

#### Approach Embankment (EA)

The south embankment is fairly level on the west side, and steep on the east. Both are in good condition. At the North Abutment BW, the embankment is fairly level and in good condition. At the North Abutment BE, the west side is moderately level, and the east side is steep but well vegetated.

#### Approach Guardrail (EA)

The guardrails are in overall good condition. There is no guardrail on the northwest of the BE approach due to slow speeds and right hand turns only into the roundabout.

## **Inspector Comments - General Appraisal**

#### Superstructure

#### **Superstructure Alignment (EA)**

The alignment of the primary superstructure members is Good. The superstructure elements were aligned as intended at the time of inspection.

# **Beams/Girders (LF)**

Steel beams typically exhibit freckling and surface corrosion at random. Due to failed expansion joints and improper drainage, the beam ends below exhibit more significant corrosion and section loss. In span 13, there is bottom flange deterioration on beam 7. In spans 18BW and 19BW, beam 6 exhibits minor section loss on each side of Expansion Joint 2BW. In span 24BE, there is minor section loss on all four beams for 2' on both sides of Expansion Joint 3BE.

# Fatigue (LF)

Other fatigue prone details are the welded cover plate ends along the beam bottom flanges. No significant deficiencies were noted at these locations. No significant deficiencies were noted along the bolted splice connections.

# Protective Coating System (SF)

Freckling corrosion is prevalent throughout the beams and steel pier caps. Below the leaking joints, beams exhibit increasing deterioration of the protective coating system. Overspray from past painting is evident along the concrete soffit.

# Diaphragms/X-Frames (EA)

Steel cross diaphragms between the steel beams exhibit light surface corrosion. Due to failed deck joints and improper drainage, the cross frames below exhibit more significant corrosion and section loss. Bottom bracing struts in spans 13, 18BW, 18BE, 19BW, and 24BE were noted to have holes or become detached due to section loss.

## **Bearing Devices (EA)**

The steel fixed and rocker bearings atop the concrete piers are in overall good condition. Multiple bearing measurements did not agree with the ambient temperature averaging around 32 degrees Fahrenheit. Other bearings exhibited significant rotation, almost beyond the limits of expansion. The fascia bearings typically exhibit the heaviest corrosion and deterioration. Pack rust at multiple bearings, including at the North Abutment BW, is limiting the movement of the bearings.

## **Pins/Hangers/Hinges (EA)**

Due to failed joint material and debris filled troughs, the hinges are constantly exposed to water and deicers. Some of the rolling hinge bearings exhibit excessive rotation compared to the ambient temperature at the time of the inspection. At Expansion Joint 2BW, the bottom hinge tooth is chipped.

#### Substructure

## Pier Columns/Bents (EA)

The reinforce concrete pier column exhibit cracking, or delamination and spalled areas. The north face of the east column at Pier 11 has a combination of delaminated and spalling concrete along the full length. Pier 13, 14, and 16 have similar spalling with exposed stirrups. The east column at Pier 15 is spalled at the top 10' H x 6' W x up to 4" D with exposed reinforcing. This deterioration was noted on the north and south faces.

#### Pier Walls (LF)

The reinforced concrete pier walls exhibit widespread cracking, spalls, and delaminations. Pier 14 has three diagonal cracks that cover the full length of the south face of the wall. These same cracks are reflected onto the north face as well. Piers 15 and 16 exhibited similar diagonal cracks. Pier 15 also has a full length longitudinal crack along the interface of the wall and the top cap.

## Abutment Walls (LF)

The South Abutment has a couple of minor spalls and is rust stained from the expansion joint. The North

Abutment BW wall has a large 6' L x 2' H x 4" D spall below bay 1 and longitudinal cracks that are wider near the middle bays. The North Abutment BE has a few superficial spalls and vertical cracks is rust stained from the expansion joint. All three abutments have minor to significant debris on the beam seats.

# Pier Caps (LF)

The reinforced concrete pier caps typically exhibit spalling along the ends and above the reinforced concrete columns. Pier 8 has spalling on both ends and also has vertical cracking along the mid-span. Very similar spalling and cracking was noted along Pier 9. At Pier 11, cracking along the south face appears to be mirrored onto the north face. On the south face of Pier 13, the full height of the end is spalled with exposed reinforcing. The spalling continues under the cap and up the north face as well. Pier 18BW has map cracks and deep delamination.

# Backwalls (LF)

The North Abutment BW backwall has multiple vertical cracks with associated diagonal cracking up to 1/8" wide. Both North Abutment backwalls exhibit staining and discoloration due to the leaking/failing expansion joint above. The South Abutment backwall has minor vertical cracks. The erosion ditch below the east corner of the abutment does not appear to be affecting the foundation at this time.

# Wingwalls (EA)

No significant deficiencies were noted during the inspection.

# **Slope Protection (EA)**

The slope protection at the north abutments are in overall good condition. The South Abutment slope protection is in good for the most part aside from below the east corner is a  $10^{\circ}W \times 5^{\circ}$  deep erosion rut.

<u>Culvert</u>

# **Inspector Comments - Waterway**

Waterway Adequacy

<u>Channel</u>

Scour Critical