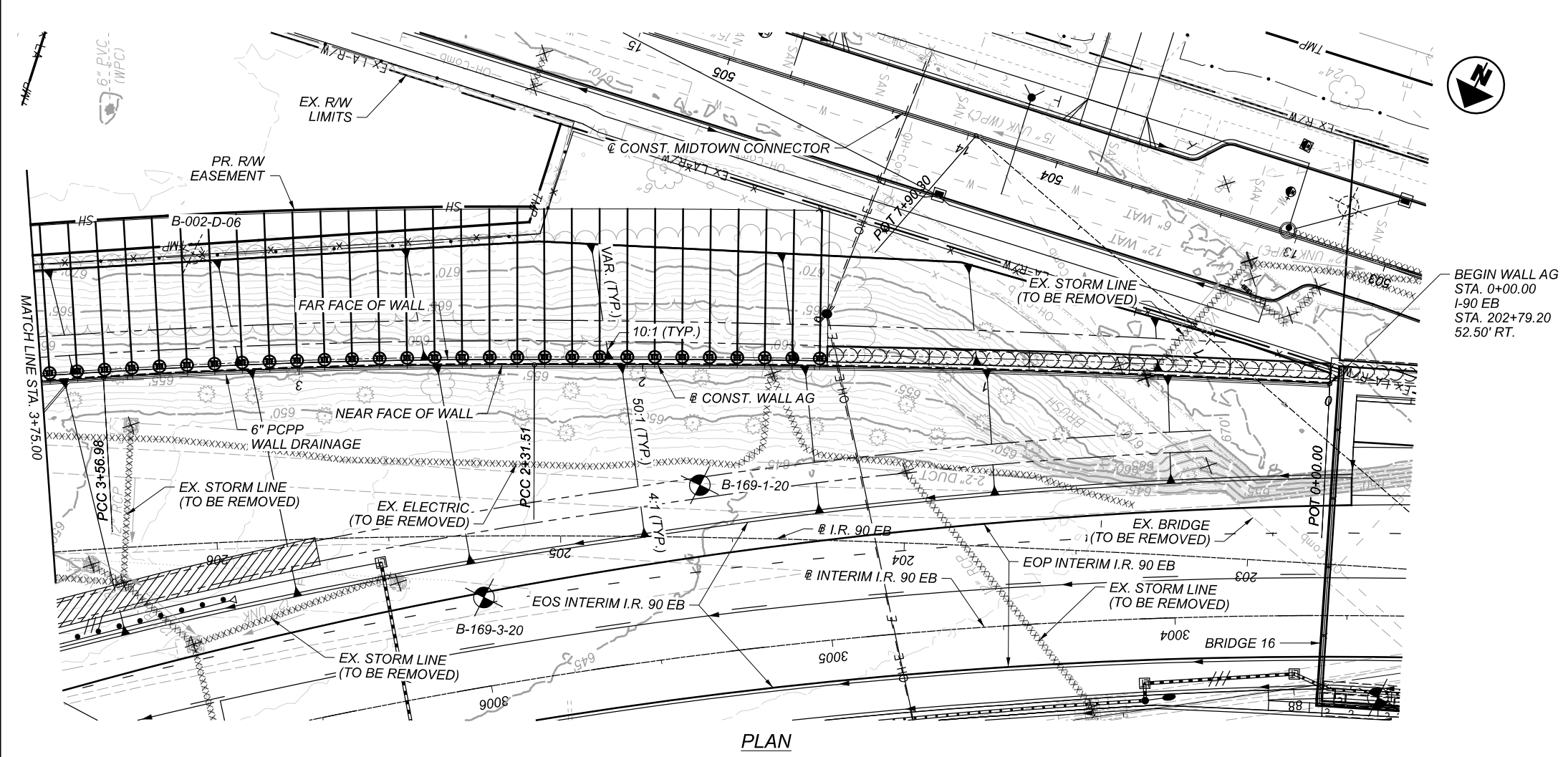
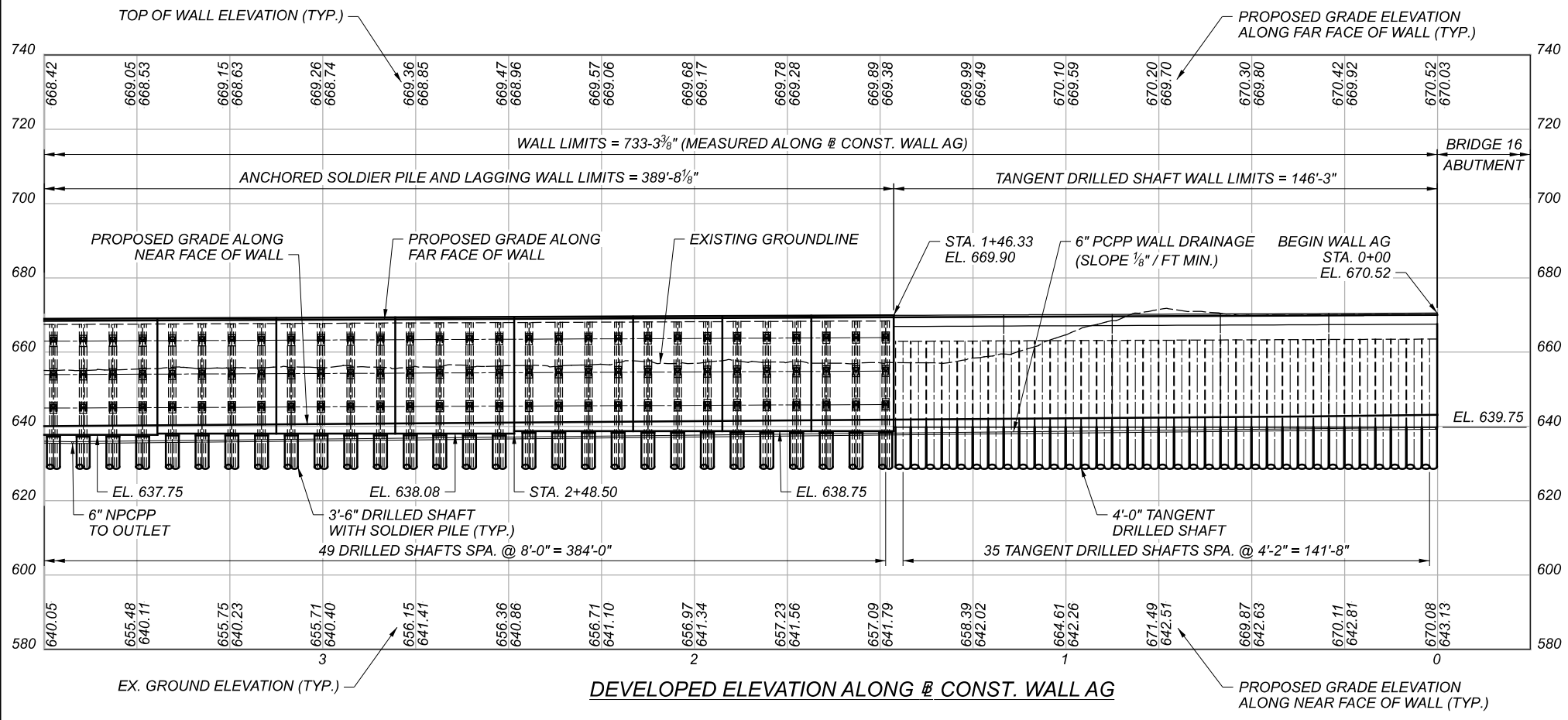


MODEL: BLP Wall AG - Wall AG Plan PAPER SIZE: 17x11 (in.) DATE: 8/11/2022 TIME: 3:20:51 PM USER: Shanae Weiss
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PLAN



DEVELOPED ELEVATION ALONG CONST. WALL AG

BENCHMARK DATA

BM #54 STA. 33+01.73	ELEV. 672.54	OFFSET 46.13 RT.	CUT CROSS
BM #62 STA. 35+23.59	ELEV. 672.11	OFFSET 1165.82 LT.	RR SPIKE
BM #72 STA. 23+49.63	ELEV. 674.06	OFFSET 52.19 LT.	CUT CROSS
BM #73 STA. 37+10.17	ELEV. 671.90	OFFSET 403.44' LT.	CUT CROSS

FOR ADDITIONAL BENCHMARK INFORMATION, SEE ROADWAY PLAN SHEET 3 / 2338

WALL AG

CURVE DATA
 P.I. = Sta. 1+15.79
 Δ = 03°18'59" LT
 Dc = 01°25'57"
 R = 3,999.81'
 T = 115.79'
 L = 231.51'
 E = 1.68'

CURVE DATA
 P.I. = Sta. 2+94.27
 Δ = 03°41'12" LT
 Dc = 02°56'18"
 R = 1,950.00'
 T = 62.76'
 L = 125.47'
 E = 1.01'

RAMP A2

SPIRAL DATA
 P.I. = Sta. 205+66.41
 Ls = 555.00'
 θs = 11°47'37"
 LT = 370.82'
 ST = 185.75'
 x = 552.65'
 y = 37.97'
 k = 277.11'
 p = 9.51'
 C = 553.96'
 Start = Sta. 201+95.58
 End = Sta. 207+50.58
 C.B. = N63°13'44"E

RAMP J2

CURVE DATA
 P.I. = Sta. 3005+92.95
 Δ = 25°41'03" LT
 Dc = 03°57'00"
 R = 1,450.53'
 T = 330.67'
 L = 650.23'
 E = 37.21'

BORING LOCATION*

BORING	STATION	OFFSET	TOP OF ROCK EL.
B-169-1-20	204+59.78	15.18' RT.	-
B-169-3-20	205+22.44	17.89' LT.	-

* = BORING STATION AND OFFSETS ARE FROM I.R. 90 EB

PROPOSED WORK

THE PROPOSED WORK CONSIST OF CONSTRUCTING RETAINING WALLS ALONG THE RIGHT (SOUTH) SIDE OF I.R. 90 EB BETWEEN BRIDGES 16 AND 13. PORTIONS OF THE WALL UTILIZE TOP DOWN CONSTRUCTION CONSISTING OF SOLDIER PILES, ANCHORS, TIMBER LAGGING AND CAST IN PLACE CONCRETE FACING. PORTIONS OF THE WALL UTILIZE TOP DOWN CONSTRUCTION CONSISTING OF TANGENT DRILLED SHAFTS AND CAST IN PLACE CONCRETE FACING.

NOTES

- EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.
- FOR WALL CROSS SECTIONS, SEE SHEETS 562 TO 569 / 2338
- STATION AND WALL OFFSETS SHOWN AT NEAR FACE OF WALL.

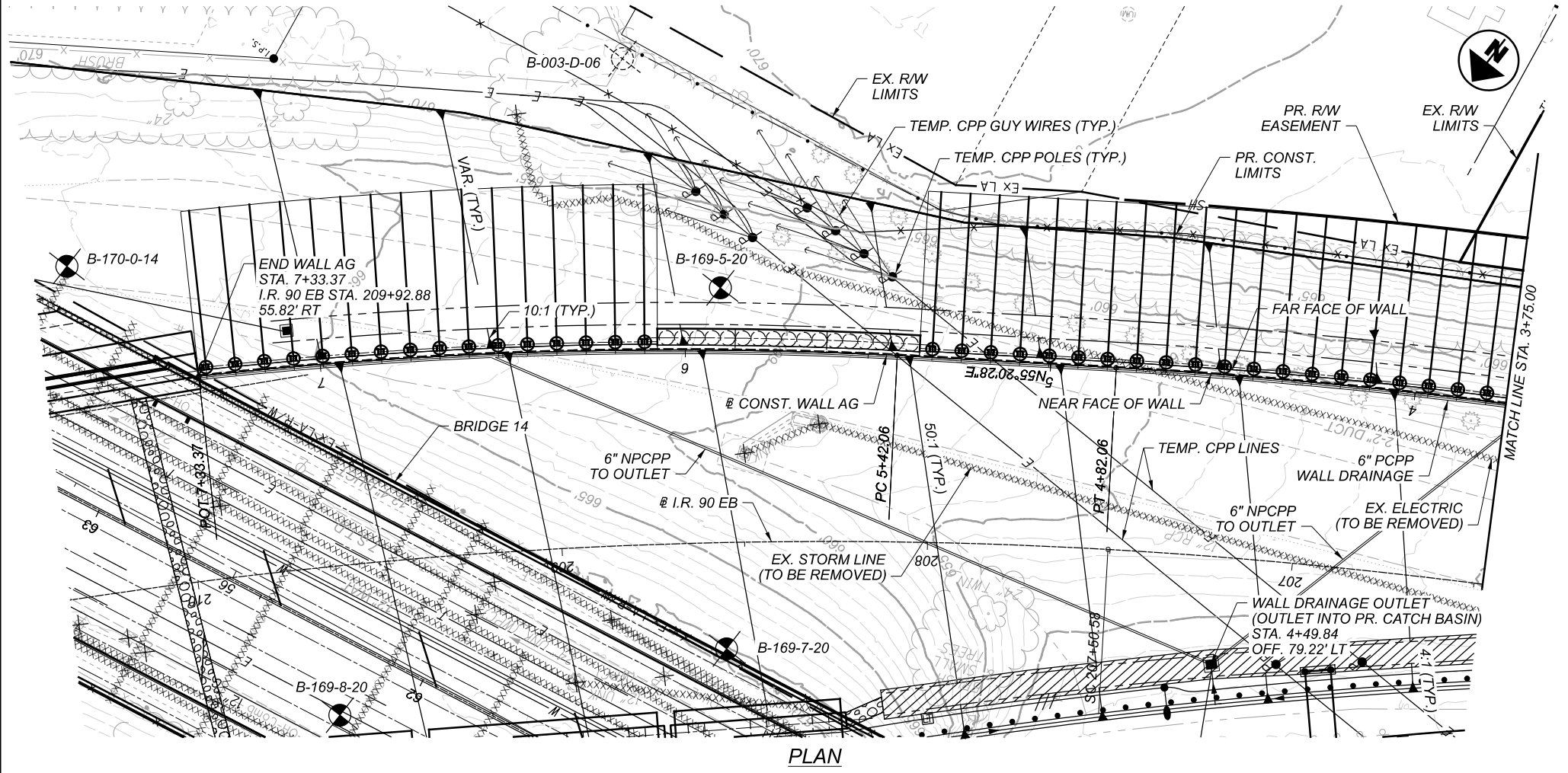
LEGEND

- ⊕ HISTORIC BORING LOCATIONS
- ⊙ PROJECT BORING LOCATIONS
- CONST. = CONSTRUCTION
- EOP = EDGE OF PAVEMENT
- EOS = EDGE OF SHOULDER

WALL PLAN AND PROFILE (1 OF 2)
WALL AG
ALONG SOUTH SIDE OF I.R. 90

DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPC 08-04-22
PROJECT ID	82382
SUBSET	1
TOTAL	18
SHEET	1038
TOTAL	2338

MODEL: BLP Wall AG - Wall AG Plans-1 PAPER SIZE: 17x11 (in.) DATE: 8/11/2022 TIME: 3:21:16 PM USER: Shane.Welks
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PLAN

BENCHMARK DATA

BM #54 STA. 33+01.73	ELEV. 672.54	OFFSET 46.13 RT.	CUT CROSS
BM #62 STA. 35+23.59	ELEV. 672.11	OFFSET 1165.82 LT.	RR SPIKE
BM #72 STA. 23+49.63	ELEV. 674.06	OFFSET 52.19 LT.	CUT CROSS
BM #73 STA. 37+10.17	ELEV. 671.90	OFFSET 403.44' LT.	CUT CROSS

FOR ADDITIONAL BENCHMARK INFORMATION, SEE ROADWAY PLAN SHEET 3 / 2338

WALL AG

CURVE DATA

P.I. = Sta. 4+19.56
 $\Delta = 04^\circ 48' 35''$ LT
 $Dc = 03^\circ 50' 43''$
 $R = 1,490.00'$
 $T = 62.58'$
 $L = 125.08'$
 $E = 1.31'$

RAMP A2

CURVE DATA

P.I. = Sta. 6+37.92
 $\Delta = 09^\circ 08' 04''$ LT
 $Dc = 04^\circ 46' 29''$
 $R = 1,200.00'$
 $T = 95.86'$
 $L = 191.31'$
 $E = 3.82'$

SPIRAL DATA

P.I. = Sta. 205+66.41
 $Ls = 555.00'$
 $\theta_s = 11^\circ 47' 37''$
 $LT = 370.82'$
 $ST = 185.75'$
 $x = 552.65'$
 $y = 37.97'$
 $k = 277.11'$
 $p = 9.51'$
 $C = 553.96'$
 Start = Sta. 201+95.58
 End = Sta. 207+50.58
 C.B. = $N63^\circ 13' 44'' E$

RAMP J2

CURVE DATA

P.I. = Sta. 3005+92.95
 $\Delta = 25^\circ 41' 03''$ LT
 $Dc = 03^\circ 57' 00''$
 $R = 1,450.53'$
 $T = 330.67'$
 $L = 650.23'$
 $E = 37.21'$

BORING LOCATION*

BORING	STATION	OFFSET	TOP OF ROCK EL.
B-169-5-20	208+55.19	69.71' RT.	-
B-169-7-20	208+55.57	29.08' LT.	-
B-169-8-20	209+65.12	40.70' LT.	-
B-170-0-14	210+22.99	90.44' RT.	-

* = BORING STATION AND OFFSETS ARE FROM @ I.R. 90 EB

PROPOSED WORK

THE PROPOSED WORK CONSIST OF CONSTRUCTING RETAINING WALLS ALONG THE RIGHT (SOUTH) SIDE OF I.R. 90 EB BETWEEN BRIDGES 16 AND 13. PORTIONS OF THE WALL UTILIZE TOP DOWN CONSTRUCTION CONSISTING OF SOLDIER PILES, ANCHORS, TIMBER LAGGING AND CAST IN PLACE CONCRETE FACING. PORTIONS OF THE WALL UTILIZE TOP DOWN CONSTRUCTION CONSISTING OF TANGENT DRILLED SHAFTS AND CAST IN PLACE CONCRETE FACING.

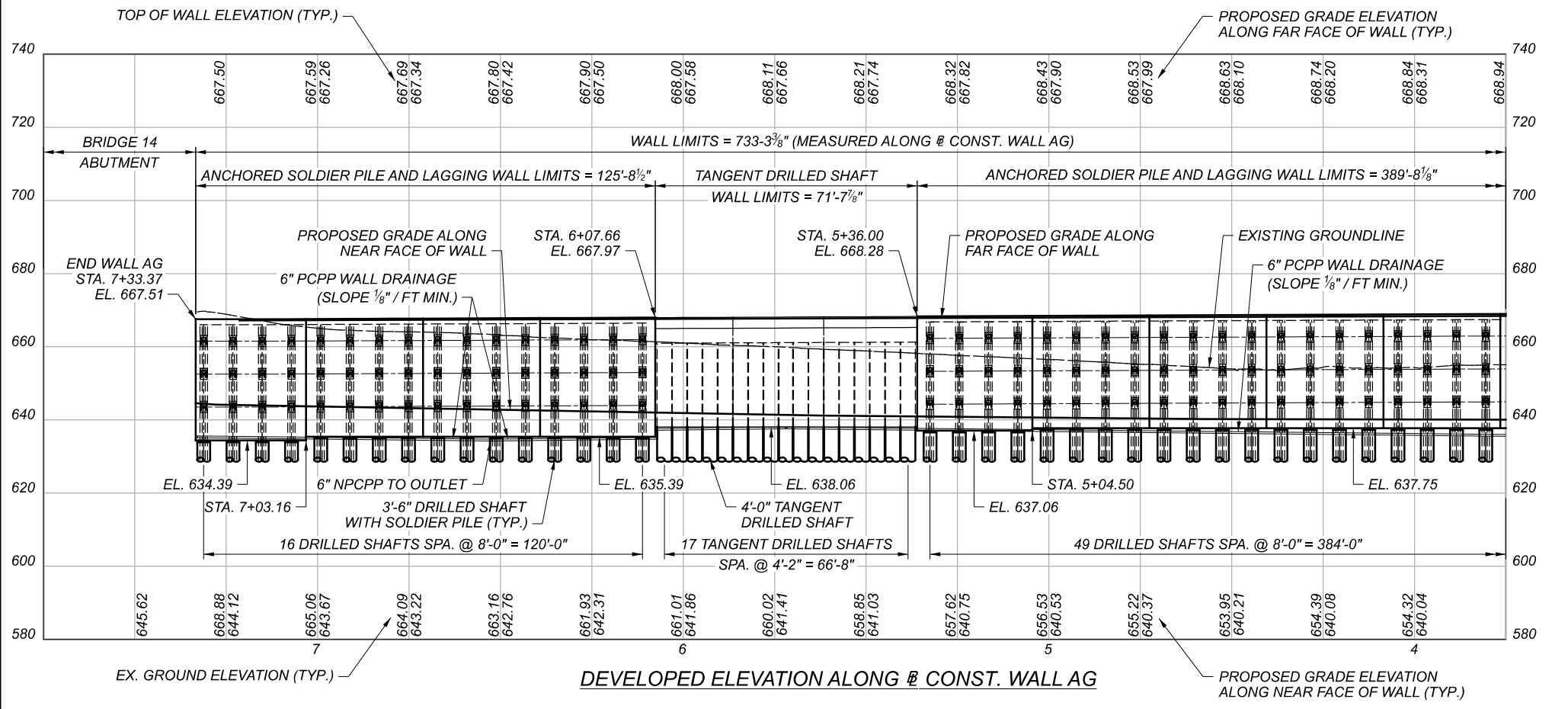
NOTES

- EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.
- FOR WALL CROSS SECTIONS, SEE SHEETS 562 TO 568 / 2338
- STATION AND WALL OFFSETS SHOWN AT FRONT FACE OF WALL.

LEGEND

- HISTORIC BORING LOCATIONS
- PROJECT BORING LOCATIONS

CONST. = CONSTRUCTION
 EOP = EDGE OF PAVEMENT
 EOS = EDGE OF SHOULDER



DEVELOPED ELEVATION @ CONST. WALL AG

SFN	N/A
DESIGN AGENCY	
DESIGNER	Michael Baker INTERNATIONAL
CHECKER	
SSW	GZ
REVIEWER	
LPC	08-04-22
PROJECT ID	82382
SUBSET	TOTAL
2	18
SHEET	TOTAL
1039	2338

STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS:

REFER TO THE FOLLOWING STANDARD DRAWINGS:

VPF-1-90 DATED 7-20-18
 DM-1.1 DATED 7-17-20

REFER TO THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS:

800 DATED 7-16-21
 866 DATED 4-21-17

AND THE FOLLOWING SUPPLEMENT:

1083 DATED 1-20-17

DESIGN SPECIFICATIONS:

THIS STRUCTURE CONFORMS TO THE 9TH EDITION OF THE "LRFD BRIDGE DESIGN SPECIFICATIONS" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 2020 AND THE ODOT BRIDGE DESIGN MANUAL, 2020.

DESIGN DATA:

CONCRETE CLASS QC1 - COMPRESSIVE STRENGTH 4.0 KSI
 (CONCRETE FACING AND PRECAST LAGGING)

CONCRETE CLASS QC5 - COMPRESSIVE STRENGTH 4.5 KSI (DRILLED SHAFTS)

REINFORCING STEEL / WELDED WIRE REINFORCEMENT - MINIMUM YIELD STRENGTH 60 KSI

STEEL SOLDIER PILES - ASTM A572 - YIELD STRENGTH 50 KSI

SEQUENCE OF CONSTRUCTION:

CONSTRUCT WALL AG DURING MOT PHASE 2.

SEE MAINTENANCE OF TRAFFIC NOTES FOR ADDITIONAL PHASES AND INFORMATION

ITEM 503 - UNCLASSIFIED EXCAVATION, AS PER PLAN

THIS WORK CONSISTS OF EXCAVATION NECESSARY TO INSTALL THE TEMPORARY TIMBER LAGGING AND ANY EMBANKMENT/EXCAVATION REQUIRED TO INSTALL A DRILLING BENCH TO INSTALL THE SOLDIER PILES. THE EXCAVATION SHALL BE PERFORMED IN ACCORDANCE WITH C&MS 503, EXCEPT AS MODIFIED AND SUPPLEMENTED BELOW.

THE SOLDIER PILE WALL SHALL BE CONSTRUCTED IN A TOP-DOWN MANNER. EXCAVATION FOR THE WALL SHALL BE MINIMIZED UNTIL ALL SOLDIER PILES ARE INSTALLED. DO NOT EXCAVATE MORE THAN 3 FEET OF THE EXISTING GRADE FOR INSTALLATION OF THE SOLDIER PILES.

REMOVE LOW STRENGTH MORTAR FROM THE PILE AS THE EXCAVATION PROGRESSES, SUFFICIENT TO PLACE THE TIMBER LAGGING AGAINST THE PILE FLANGE. FOLLOW THE EXCAVATION CLOSELY WITH PLACEMENT OF THE TIMBER LAGGING. LIMIT HEIGHT OF UNSUPPORTED, EXPOSED FACE OF SOIL TO 3 FEET OR LESS AS REQUIRED BY LOCAL SITE CONDITIONS DURING EXCAVATION.

CAREFULLY PERFORM EXCAVATION FOR THE INSTALLATION OF LAGGING TO MINIMIZE THE FORMATION OF VOIDS. BACKPACK VOIDS BETWEEN SOIL AND LAGGING WITH NUMBER 57 STONE PER C&MS TABLE 703.01-1 OR APPROVED EQUAL TO THE SATISFACTION OF THE ENGINEER.

EXCAVATION SHALL PROCEED AT A PACE THAT PREVENTS MOVEMENT OF THE WALL AND LOSS OF GROUND. WHEN UNSTABLE MATERIAL IS ENCOUNTERED DURING EXCAVATION, TAKE NECESSARY ACTIONS TO STABILIZE THE MATERIAL AND PREVENT GROUND DISPLACEMENT.

BEFORE PLACING LAGGING, SMOOTH THE SOIL FACE TO CREATE A CONTACT SURFACE FOR THE LAGGING. FLOWABLE FILL SHALL BE PLACED TO FILL LARGE VOIDS BEHIND THE LAGGING. MAINTAIN A GAP OF ¼ INCH TO ½ INCH BETWEEN EACH HORIZONTAL LAGGING BOARD FOR DRAINAGE BETWEEN ADJACENT LAGGING TIMBERS.

PAYMENT FOR LABOR, EQUIPMENT AND MATERIALS FOR THE WORK OUTLINED ABOVE SHALL BE INCLUDED IN THE CUBIC YARD CONTRACT PRICE FOR ITEM 503 UNCLASSIFIED EXCAVATION, AS PER PLAN.

ITEM 507 - STEEL PILES, MISC.: SOLDIER PILES W 16X50

THIS WORK CONSISTS OF FURNISHING AND PLACING STEEL SOLDIER PILES INTO DRILLED HOLES AND INCLUDES MONITORING PLUMBNESS. FURNISH SOLDIER PILES CONSISTING OF STRUCTURAL STEEL MEMBERS THAT MEET THE REQUIREMENTS AND CONFORM TO ASTM A572 GRADE 50. DO NOT FIELD WELD OR SPLICE STEEL SOLDIER PILES. THE ANCHOR DETAIL WHERE THE TIE BACK ANCHOR PASSES THROUGH THE FLANGES AND WEB SHALL BE SHOP FABRICATED AND NOT BE WELDED ON SITE.

MEASUREMENT FOR PAYMENT WILL BE THE DISTANCE FROM THE TOP OF THE PILE TO THE BOTTOM OF THE DRILLED SHAFT, AS DETERMINED BY THE ENGINEER. PAYMENT IS FULL COMPENSATION FOR FURNISHING AND PLACING THE SOLDIER PILES AND MONITORING THEIR PLUMBNESS UNTIL PLACEMENT OF THE CONCRETE FACING HAS BEGUN. THE DEPARTMENT WILL PAY FOR SOLDIER PILES AT THE CONTRACT UNIT PRICE PER FOOT OF ITEM 507 - STEEL PILES, MISC., SOLDIER PILES HP14X73.

ITEM 509 - WALL FACING REINFORCEMENT

THE CONTRACTOR MAY REPLACE THE REINFORCING BARS IN THE RETAINING WALL FACING WITH EPOXY COATED WELDED WIRE FABRIC CONFORMING TO C&MS 709.14. THE EPOXY COATED WELDED WIRE FABRIC MUST PROVIDE AN EQUIVALENT AREA OF STEEL IN EACH DIRECTION AS THE REINFORCING BARS SHOWN IN THE PLANS.

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, RETAINING/WINGWALL NOT INCLUDING FOOTING, AS PER PLAN

THIS ITEM CONSISTS OF CONSTRUCTING A CAST IN PLACE REINFORCED CONCRETE WALL ON THE FRONT SIDE OF THE SOLDIER PILES TO SERVE AS PERMANENT FACING OVER THE TEMPORARY TIMBER LAGGING.

STEEL STUDS SHALL BE WELDED ACCORDING TO C&MS 513.22 AND AS SHOWN ON THE PLANS TO THE FRONT FLANGES OF THE SOLDIER PILES FOR CONNECTION OF THE REINFORCED CONCRETE FACING. STUDS SHALL BE SPACED AT 12 INCHES VERTICALLY STARTING 6 INCHES BELOW THE TOP OF THE PILE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL WELDED AND ACCEPTED STEEL STUDS AGAINST DAMAGE UNTIL THE PERMANENT CAST IN PLACE REINFORCED CONCRETE FACING IS CAST. ALL DAMAGED STUDS SHALL BE REPLACED AT NO EXTRA COST TO THE DEPARTMENT.

FORM LINERS (ARCHITECTURAL TREATMENT WITH AESTHETIC PATTERNS) SHALL BE PROVIDED AS SHOWN ON THE LANDSCAPING PLANS.

HORIZONTAL JOINTS ARE PROHIBITED IN THE CAST IN PLACE CONCRETE FACING.

PAYMENT SHALL INCLUDE ALL LABOR, MATERIALS AND INCIDENTALS NECESSARY TO FURNISH AND PLACE CONCRETE FOR THE CAST IN PLACE FACING, INCLUDING ½ INCH AND 1 INCH PREFORMED EXPANSION JOINT FILLER, DOWELS AT JOINTS, BOND BREAKER AT DOWELS AT EXPANSION JOINTS, NON-BITUMINOUS JOINT SEALER AT THE JOINTS, WELDED STUD CONNECTORS AND FORM LINERS

ITEM 512 - SEALING OF CONCRETE SURFACES, AS PER PLAN (PERMANENT GRAFFITI PROTECTION)

APPLY A PERMANENT GRAFFITI COATING MEETING THE REQUIREMENTS OF SUPPLEMENT 1083. THE GRAFFITI COATING MUST BE COMPATIBLE WITH THE UNDERLYING CONCRETE SEALER. APPLY THE GRAFFITI COATING ACCORDING TO THE MANUFACTURE'S REQUIREMENTS. THE ADDITIONAL MATERIAL AND LABOR REQUIRED TO SEAL THE FORM LINER RELIEF SHALL BE INCLUDED IN THIS ITEM. TO ACCOUNT FOR THE SURFACE VARIATIONS DUE TO THE FORM LINERS, AN EXTRA 20 PERCENT HAS BEEN ADDED TO THE SEALING QUANTITIES FOR THE PURPOSE OF ESTIMATING. PROVIDE A COATING THAT MEETS THE REQUIREMENTS LISTED BELOW.

- A. THE MATERIAL SHALL BE A SINGLE COMPONENT, RTV (ROOM TEMPERATURE VULCANIZED) NEUTRAL MOISTURE CURE, PERMANENT (NON-SACRIFICIAL), TYPE III (WATER CLEANABLE) POLYSILOXANE (SILICONE) ANTI-GRAFFITI COATING, FREE OF ANY WAXES, EPOXIES OR POLYURETHANE COMPONENTS.
- B. THE COATING SHALL BE A ONE COAT SYSTEM (NO PRIMER) CAPABLE OF BEING SPRAY APPLIED TO A DRY FILM THICKNESS OF 15 MILS (375 MICRONS) WITHOUT RUNS OR SAGS (MULTIPLE COAT APPLICATION ACCEPTABLE FOR BRUSH/ROLLER USAGE AND PRIMER USAGE ACCEPTABLE FOR SPECIALTY SUBSTRATES SUCH AS GALVANIZED METAL).
- C. THE COATING SHALL EMIT LESS THAN 300 G/L (2.5 POUNDS PER GALLON) OF VOLATILE ORGANIC COMPOUNDS (EPA METHOD 24).

ITEM 512 - SEALING OF CONCRETE SURFACES, AS PER PLAN (PERMANENT GRAFFITI PROTECTION) (CONTINUED)

D. THE COATING SHALL MEET THE FOLLOWING PERFORMANCE REQUIREMENTS:

- 1. CLEANABILITY LEVEL 1 (GRAFFITI COMPLETELY REMOVED WITH COLD WATER POWER WASH) AS PER ASTM D7089 WITH LOW PRESSURE (1200 PSI) COLD WATER WASH AFTER 2000 HOURS ACCELERATED UV-CONDENSATION EXPOSURE IN ACCORDANCE WITH ASTM D4587.
- 2. GRAFFITI RESISTANCE LESS THAN 7.5 AS PER ASTM D6578 AFTER 2000 HOURS ACCELERATED UV-CONDENSATION EXPOSURE IN ACCORDANCE WITH ASTM D4587.
- 3. NO SIGNS OF GRAFFITI OR GRAFFITI STAINING AND MUST BE INTACT AND EXHIBIT NO SIGNS OF STREAKING, CRACKING, PINHOLING, DISCOLORING OR OTHER VISIBLE COATING DEGRADATION UPON CASUAL OBSERVATION WHEN TESTED IN ACCORDANCE WITH TXDOT TEX 890-B, TYPE III METHOD.
- 4. BREATHABILITY OF 10 PERMS (+/- 3) PER ASTM D1653 USING "WET CUP METHOD".
- 5. ELONGATION AT BREAK GREATER THAN 100% AS PER ASTM D412 (USING DIE "D").
- 6. ADHESION RATING OF "8 - DIFFICULT TO REMOVE" AS PER ASTM D6677 (ADHESION BY KNIFE).

ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE), AS PER PLAN

APPLY SEALER TO ALL EXPOSED SURFACES INCLUDING THE CAST IN PLACE CONCRETE FACING. THE FINISH COAT COLOR SHALL BE AS REQUIRED BY THE LANDSCAPING PLANS. SEE LANDSCAPING PLANS FOR COLOR INFORMATION. THE ADDITIONAL MATERIAL AND LABOR REQUIRED TO SEAL THE FORM LINER RELIEF SHALL BE INCLUDED IN THIS ITEM. TO ACCOUNT FOR THE SURFACE VARIATIONS DUE TO THE FORM LINERS, AN EXTRA 20 PERCENT HAS BEEN ADDED TO THE SEALING QUANTITIES FOR THE PURPOSE OF ESTIMATING.

ITEM 512 - TYPE 2 WATERPROOFING, AS PER PLAN

TYPE 2 WATERPROOFING SHALL BE ATTACHED TO THE WOOD LAGGING, CENTERED AT ALL EXPANSION AND CONTRACTION JOINTS VERTICALLY FROM THE TOP OF THE TIMBER LAGGING DOWN TO THE TOP OF THE POROUS BACKFILL. TYPE 2 WATERPROOFING SHALL ALSO BE ATTACHED TO THE BACK SIDE OF THE CAST IN PLACE CONCRETE FACING AND COPING ABOVE THE TIMBER LAGGING AT ALL EXPANSION AND CONTRACTION JOINTS.

PAYMENT FOR ALL LABOR, EQUIPMENT AND MATERIAL FOR THE WORK OUTLINED ABOVE SHALL BE INCLUDED IN THE PAYMENT PER SQUARE YARD CONTRACT PRICE FOR ITEM 512, TYPE 2 WATERPROOFING, AS PER PLAN.

ITEM 513 - WELDED STUD SHEAR CONNECTORS

SOLDIER PILES WHICH REQUIRE HEADED STUDS ARE SHOWN IN THE TABLE ON THE TYPICAL SECTION SHEETS FOR EACH WALL. WELD HEADED STEEL STUDS TO THE FLANGES OF THE SOLDIER PILE IN ORDER TO CONNECT THE CONCRETE WALL FACING TO THE SOLDIER PILE. ATTACH HEADED ¾" DIAMETER STUDS ACCORDING TO C&MS 513.22 AND AS SHOWN IN THE PLANS. THE CONTRACTOR MAY ATTACH THE STUDS EITHER BEFORE PLACING THE SOLDIER PILE IN THE DRILLED HOLE OR AFTER EXCAVATING IN FRONT OF THE WALL. PROTECT THE HEADED STUDS FROM DAMAGE UNTIL THE CONCRETE WALL FACING IS POURED. REPAIR OR REPLACE DAMAGED HEADED STUDS AT NO EXPENSE TO THE DEPARTMENT.

WALL GENERAL NOTES (1 OF 3)
 WALL AG
 ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC 08-04-22	
PROJECT ID	
82382	
SUBSET	TOTAL
3	18
SHEET	
1040	2338

ITEM 518 - PREFABRICATED GEOCOMPOSITE DRAIN

THIS WORK CONSISTS OF FURNISHING AND PLACING PREFABRICATED GEOCOMPOSITE DRAIN (PGD) AGAINST THE TIMBER LAGGING OR AGAINST THE CONCRETE WALL FACING WHERE THE TIMBER LAGGING IS NOT PRESENT.

FURNISH PGD CONSISTING OF A DRAINAGE CORE WITH A GEOTEXTILE FABRIC BONDED TO AT LEAST ONE SIDE. USE CORE MATERIAL THAT CONSISTS OF A STABLE, POLYMER PLASTIC MATERIAL WITH A CUSPATED OR GEONET STRUCTURE. THE CORE MATERIAL SHALL HAVE SUFFICIENT FLEXIBILITY TO WITHSTAND BENDING AND HANDLING DURING INSTALLATION WITHOUT DAMAGE. FURNISH GEOTEXTILE COMPOSED OF STRONG ROT-PROOF POLYMERIC FIBERS FORMED INTO A WOVEN OR NON-WOVEN FABRIC. FURNISH PGD CONFORMING TO THE FOLLOWING REQUIREMENTS. FURNISH MANUFACTURER'S CERTIFIED TEST DATA.

	PROPERTY	TEST METHOD	VALUE
CORE	THICKNESS	ASTM D5199	0.4 INCH
	COMPRESSIVE STRENGTH	ASTM D1621	13,650 PSF MIN.
	FLOW RATE	ASTM D4716	9 TO 25 GPM/FT.
FABRIC	APPARENT OPENING SIZE	ASTM D4751	0.3 MM MAX.
	FLOW RATE	ASTM D4491	40 GPM/ SQ.FT. MIN.
	GRAB TENSILE STRENGTH	ASTM D4632	90 LBS MIN.
	CBR PUNCTURE	ASTM D6241	65 LBS MIN.

PLACE PGD BETWEEN THE SOLDIER PILES, INCLUDING THE CANTILEVERED PORTION AT THE END OF THE WALL. PLACE THE SIDED FACED WITH THE GEOTEXTILE AGAINST THE TIMBER LAGGING, FACING TOWARDS THE RETAINED GROUND, AND SECURE THE PGD TO THE LAGGING. USE NAILS AND WASHERS AT LEAST 1 INCH DIAMETER IN SIZE TO SECURE THE PGD ALONG ITS EDGES AT A MAXIMUM SPACING OF 4 FEET.

SPLICE ABUTTING SECTIONS TOGETHER BY OVERLAPPING THE GEOTEXTILE FLAP (IF PROVIDED) ON ONE SECTION WITH THE ADJACENT SECTION OF PGD. OVERLAP THE GEOTEXTILE IN A SHINGLED OVERLAP SO THAT THE UPPER GEOTEXTILE IS ON TOP OF THE LOWER GEOTEXTILE. IF A GEOTEXTILE FLAP IS NOT PROVIDED, COVER THE SEAM WITH A 12 INCH WIDE STRIP OF GEOTEXTILE FABRIC CENTERED OVER THE SEAM AND SECURED IN PLACE USING 3 INCH WIDE WATERPROOF PLASTIC TAPE.

SEAL ALL EXPOSED EDGES OF THE CORE MATERIAL TO PREVENT SOIL INTRUSION. SEAL EXPOSED EDGES BY FOLDING THE GEOTEXTILE FLAPS OVER AND AROUND THE PGD OR, IF A FLAP IS NOT PROVIDED, COVERING THE EXPOSED EDGE WITH A 12 INCH WIDE STRIP OF GEOTEXTILE FABRIC, TAPING THE STRIP TO THE PGD GEOTEXTILE 8 INCHES FROM THE EXPOSED EDGE, AND FOLDING THE REMAINING 4 INCHES OVER AND AROUND THE PGD. SECURE LOOSE EDGES OF THE GEOTEXTILE FABRIC WITH 3 INCH WIDE WATERPROOF PLASTIC TAPE.

REPAIR ANY DAMAGE TO THE GEOTEXTILE FABRIC BY COVERING WITH A PATCH WHICH OVERLAPS THE DAMAGED AREA AND EXTENDS AT LEAST 6 INCHES BEYOND THE EDGE OF THE DAMAGED AREA. TAPE THE EDGES OF THE PATCH IN PLACE USING 3 INCH WIDE WATERPROOF PLASTIC TAPE. IF THE CORE OF THE PGD IS DAMAGED, REPLACE IT WITH A NEW SECTION OF PGD AND SPLICE AS DESCRIBED ABOVE.

WHERE SHOWN ON THE PLANS, PLACE THE BOTTOM OF THE PGD ADJACENT TO A PERFORATED DRAINAGE COLLECTION PIPE AND POROUS BACKFILL AND COVER WITH GEOTEXTILE FABRIC. ENSURE A CONTINUOUS DRAINAGE PATH FROM THE PGD CORE TO THE PIPE.

IF TIMBER LAGGING IS NOT REQUIRED BECAUSE THE PORTION OF THE WALL IS ABOVE THE EXISTING GROUND, ATTACH THE PGD TO THE BACK FACE OF THE CONCRETE WALL FACING UNTIL BACKFILL IS PLACED.

ITEM 524 - DRILLED SHAFTS, 42" DIAMETER, ABOVE BEDROCK, AS PER PLAN

THIS WORK CONSISTS OF FURNISHING AND INSTALLING DRILLED SHAFTS FOR SOLDIER PILE WALLS. THE DRILLED SHAFTS ARE REINFORCED WITH SOLDIER PILES INSTEAD OF REINFORCING STEEL CAGES. THE SOLDIER PILES EXTEND ABOVE THE TOP OF THE DRILLED SHAFT. FURNISH AND INSTALL THE DRILLED SHAFTS ACCORDING TO C&MS 524 EXCEPT AS MODIFIED AND SUPPLEMENTED BELOW.

EXCAVATE THE HOLE FOR THE DRILLED SHAFT WITHIN 3 INCHES OF THE PLAN LOCATION. PLACE THE SOLDIER PILE WITHIN THE HOLE SO IT IS VERTICAL. PLACE THE SOLDIER PILE SO THAT THE FLANGES ARE PARALLEL TO THE CENTERLINE OF THE ROW OF DRILLED SHAFTS. DO NOT ALLOW THE ORIENTATION OF THE FLANGES TO VARY BY MORE THAN 10 DEGREES. SUPPORT THE SOLDIER PILE SO THAT IT DOES NOT MOVE DURING CONCRETE PLACEMENT.

USE CLASS QC5 CONCRETE ACCORDING TO C&MS 511. PLACE CONCRETE TO THE ELEVATION FOR THE TOP OF THE DRILLED SHAFT. THE CONTRACTOR MAY PLACE CONCRETE USING THE FREE FALL METHOD PROVIDED THE DEPTH OF WATER IS LESS THAN 6 INCHES AND THE CONCRETE FALLS WITHOUT STRIKING THE SIDES OF THE HOLE. POURING CONCRETE ALONG THE WEB OF THE SOLDIER PILE IS ACCEPTABLE.

CHECK THE POSITION, THE VERTICAL ALIGNMENT AND ORIENTATION OF THE SOLDIER PILE IMMEDIATELY AFTER CONCRETE PLACEMENT. MAKE CORRECTIONS AS NECESSARY TO MEET THE ABOVE TOLERANCES. FILL THE HOLE ABOVE THE CONCRETE TO THE EXISTING GROUND SURFACE WITH ITEM 613 LOW STRENGTH MORTAR BACKFILL (LSM).

REMOVE CONCRETE AND LSM AS NECESSARY FROM AROUND THE SOLDIER PILE IN ORDER TO PLACE THE LAGGING. PLACE LAGGING SO THAT THE SOLDIER PILE FLANGE OVERLAPS THE END OF THE LAGGING BY AT LEAST 3 INCHES AT BOTH ENDS OF THE LAGGING. WAIT AT LEAST 12 HOURS AFTER PLACING CONCRETE BEFORE PLACING LAGGING.

SEQUENCE OF INSTALLATION: THE INSTALLATION SEQUENCE SHALL BE SUCH THAT NO DRILLED SHAFT IS INSTALLED ADJACENT TO EITHER AN OPEN DRILLED SHAFT EXCAVATION OR A DRILLED SHAFT IN WHICH THE CONCRETE HAS LESS THAN A 48-HOUR CURE. INSTALLING THE SHAFTS IN AN ALTERNATING SEQUENCE OR ANY OTHER SEQUENCE THAT MEETS THESE CRITERIA IS PERMISSIBLE.

PROTECTION OF UNATTENDED OPEN SHAFTS: CARE SHALL BE EXERCISED AS TO COVER UNATTENDED OPEN SHAFTS. TEMPORARY COVERS SHALL BE OF ADEQUATE STRENGTH TO PREVENT A PERSON OR ANIMAL FROM FALLING IN. NO DRILLED SHAFT EXCAVATION SHALL BE LEFT UN-POURED OVERNIGHT.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MEANS AND METHODS USED TO CONSTRUCT THE DRILLED SHAFTS AND PLACE LAGGING. ANY TEMPORARY GRADING, EXCAVATION, EMBANKMENT, AGGREGATE, DRAINAGE, SHEETING, ETC. NEEDED TO COMPLETE THE WORK SHALL BE INCLUDED IN THE BID PRICE FOR THE DRILLED SHAFTS. THE COST OF ANY EXCAVATION AND SUBSEQUENT REPLACEMENT OF EMBANKMENT (PER ITEM 203 EMBANKMENT) SHALL BE INCLUDED IN THE VARIOUS BID ITEMS FOR THE DRILLED SHAFTS AND LAGGING, UNLESS SEPARATELY ITEMIZED. NO SEPARATE PAYMENT WILL BE MADE.

METHOD OF MEASUREMENT: THE DEPARTMENT WILL MEASURE DRILLED SHAFTS ABOVE BEDROCK, AS PER PLAN, ALONG THE AXIS OF THE DRILLED SHAFT FROM THE EXISTING GROUND SURFACE TO THE TOP OF BEDROCK, AS DETERMINED BY THE ENGINEER. THE DEPARTMENT WILL MEASURE DRILLED SHAFTS INTO BEDROCK, AS PER PLAN, ALONG THE AXIS OF THE DRILLED SHAFT FROM TOP OF BEDROCK TO THE BOTTOM OF THE DRILLED SHAFT, AS DETERMINED BY THE ENGINEER.

PAYMENT IS FULL COMPENSATION FOR CONSTRUCTING THE DRILLED SHAFTS, INCLUDING FURNISHING AND PLACING CONCRETE AND LSM, REMOVAL OF CONCRETE OR LSM FROM AROUND THE SOLDIER PILE IN ORDER TO PLACE LAGGING.

**ITEM 524 - DRILLED SHAFTS, 42" DIAMETER, AS PER PLAN
ITEM 524 - DRILLED SHAFTS, 48" DIAMETER, AS PER PLAN**

GENERAL:

THE CONTRACTOR SHALL PROVIDE ALL LABOR, MATERIALS AND EQUIPMENT TO INSTALL DRILLED SHAFTS AS DETAILED IN THE PLANS IN ACCORDANCE WITH THE REQUIREMENTS OF ODOT C&MS SECTION 524, AND WITH THE ADDITIONAL REQUIREMENTS DEFINED BELOW.

ANTICIPATED DRILLED SHAFT DEFLECTIONS:

TANGENT DRILLED SHAFTS ("SHAFT", "SHAFTS") ARE INCORPORATED AS WALLS IN VARIOUS STRUCTURAL ELEMENTS FOR THIS BRIDGE. AS DESIGNED AND DETAILED THE SHAFTS ARE EXPECTED TO DEFLECT UNDER THE APPLIED PERMANENT LOADS (DC, DW, EP, WA) AND TRANSIENT LOADS (LL, LS, TU) AT THE SERVICE LIMIT STATE. MEASURES FOR ACCOMODATING THESE DEFLECTIONS ARE DETAILED BELOW.

THE SHAFT HEAD IS CONSIDERED TO BE THE DESIGN BEAM SEAT ELEVATION FOR SHAFTS INCORPORATED IN ABUTMENTS. THE SHAFT HEAD IS CONSIDERED TO BE THE DESIGN TOP OF CAP ELEVATION FOR SHAFTS INCORPORATED IN RETAINING WALLS. IN BOTH CASES THE FINISHED TOP OF SHAFT IS LOWER THAN THE SHAFT HEAD ELEVATION.

THE ANTICIPATED DEFLECTION AT THE SHAFT HEAD ELEVATION RELATIVE TO THE SHAFT TIP ELEVATION DUE TO PERMANENT LOADS ARE AS FOLLOWS:

WALL AG 3.25 INCHES

DESIGN ASSUMPTIONS:

BEHAVIOR OF THE DRILLED SHAFTS AS DESCRIBED ABOVE IS PREDICATED UPON THE FOLOWING DESIGN ASSUMPTIONS:

- DESIGN HEIGHT OF DRILLED SHAFT IS THE DISTANCE FROM THE SHAFT HEAD ELEVATION TO THE DREDGE LINE ELEVATION
- PERMANENT LOAD DEFLECTIONS ARE ASSUMED TO OCCUR FOLLOWING REMOVAL OF SOIL IN FRONT OF THE TANGENT SHAFT WALLS
- ADDITIONAL ASSUMPTIONS AND CONSTRAINTS ARE DETAILED IN THE PLANS.

DREDGE LINE ELEVATIONS:

WALL AG ELEV. 638.25 & ELEV. 636.56
BOTTOM OF LEVELING PAD FOR CAST-IN-PLACE FACING

DRILLED SHAFT LOCATION SURVEY:

THE CORRECT LOCATION OF SHAFT IS CRITICAL TO ESTABLISHING AND MAINTAINING THE STRUCTURE GEOMETRY. THE CONTRACTOR SHALL EMPLOY THE SERVICES OF A OHIO REGISTERED PROFESSIONAL SURVEYOR ("THE SURVEYOR") TO ESTABLISH, MAINTAIN AND VERIFY HORIZONTAL AND VERTICAL SHAFT GEOMETRY. THE SURVEYOR SHALL BE READILY AVAILABLE TO ESTABLISH GEOMETRIC CONTROL AND PERFORM THE SURVEYS REQUIRED BELOW.

CONTRACTOR'S INSTALLATION PLAN:

THE CONTRACTOR SHALL PROVIDE AN INSTALLATION PLAN AS REQUIRED BY ODOT C&MS SECTION 524.03. THE INSTALLATION PLAN SHALL ALSO INCLUDE:

- CONTRACTOR'S PROPOSED METHODS TO MAINTAIN LOCATION AND ALIGNMENT OF SHAFTS
- CONTRACTOR'S PROPOSED METHODS FOR PERFORMING THE DRILLED SHAFT LOCATION SURVEY

COSTRUCTION CONSTRAINTS:

THE CONTRACTOR IS ADVISED THAT THE PROPOSED DRILLED SHAFT INSTALLATIONS MAY REQUIRE ADVANCING SHAFTS THROUGH EXISTING PILES. ADDITIONAL INFORMATION AND NOTES REGARDING POSSIBLE CONFLICTS ARE PROVIDED IN THE PLANS.

MATERIALS:

CONCRETE AND REINFORCING STEEL FOR DRILLED SHAFTS SHALL CONFORM TO ODOT C&MS SECTION 524.02.

A SELF CONSOLIDATION CONCRETE MIX SHALL BE INCORPORATED

THE MAXIMUM COARSE AGGREGATE SIZE SHALL BE: 1"

PERMANENT STEEL CASINGS SHALL BE ASTM A252 GRADE 3 WITH A MINIMUM YIELD STRESS OF 45 KSI. CASING SECTION LENGTHS SHALL BE MAXIMIZED TO MINIMIZE THE NUMBER OF FIELD SPLICE LOCATIONS. FIELD SPLICE LOCATIONS SHALL BE AS REQUIRED BY THE PLAN DETAILS. THE USE OF SPIRAL WELDED PIPE IS PERMITTED.

ITEM 524 - DRILLED SHAFTS, MISC.: DEMONSTRATION DRILLED SHAFT

PART 1: DESCRIPTION

THIS WORK CONSISTS OF ALL LABOR, MATERIALS, EQUIPMENT AND INCIDENTALS TO CONSTRUCT A DEMONSTRATION DRILLED SHAFT FOR TESTING AND EVALUATION TO VERIFY THE PROPOSED CONSTRUCTION METHODS FOR THE PRODUCTION OF DRILLED SHAFTS.

PART 2: MATERIALS

THE DEMONSTRATION DRILLED SHAFT SHALL USE THE SAME CONCRETE MIX DESIGN AND STEEL REINFORCEMENT AS THE PRODUCTION DRILLED SHAFTS.

PART 3: EXECUTION

SUBMIT A DRILLED SHAFT INSTALLATION PLAN TO THE ENGINEER FOR ACCEPTANCE IN ACCORDANCE WITH THE REQUIREMENTS OF C&MS 524.03. CONSTRUCT AT LEAST ONE DEMONSTRATION DRILLED SHAFT IN THE AREA SHOWN ON THE PLANS AND IN ACCORDANCE WITH THE ACCEPTED WRITTEN INSTALLATION. UPON CONSTRUCTION OF THE DEMONSTRATION DRILLED SHAFT, AND RECEIPT OF TESTING AND EVALUATION RESULTS CONFIRMING THE DEMONSTRATION DRILLED SHAFT HAS BEEN INSTALLED IN ACCORDANCE WITH CONTRACT DOCUMENT, THE ENGINEER WILL ISSUE A LETTER ACCEPTING THE INSTALLATION PLAN FOR THE CONSTRUCTION OF THE SUBSEQUENT PRODUCTION DRILLED SHAFTS.

IF MODIFICATION(S) TO THE INSTALLATION PLAN ARE MADE, WHETHER DUE TO THE TESTING AND EVALUATION RESULTS OR FOR OTHER REASON, THE DEPARTMENT WILL REQUIRE CONSTRUCTION OF AN ADDITIONAL DEMONSTRATION SHAFT CONSTRUCTED IN ACCORDANCE WITH THE MODIFIED INSTALLATION PLAN, AT NO ADDITIONAL COST. THE DIAMETER, LENGTH, REINFORCING, INSTALLATION METHODS, AND OTHER MISCELLANEOUS DETAILS OF THE DEMONSTRATION SHAFT SHALL BE THE SAME AS THE PRODUCTION DRILLED SHAFTS.

SUBMIT THE LOCATION OF THE DEMONSTRATION SHAFT TO THE ENGINEER FOR ACCEPTANCE. LOCATE THE DEMONSTRATION DRILLED SHAFT SUCH THAT NO INTERFERENCE OCCURS WITH THE FOUNDATIONS OF EXISTING OR PROPOSED STRUCTURES, THE PROPOSED MAINTENANCE OF TRAFFIC, OR EXISTING OR PROPOSED UTILITIES.

LOCATE THE DEMONSTRATION DRILLED SHAFT SO THAT TESTING DOES NOT DAMAGE THE JUVENILE JUSTICE CENTER BUILDING.

TEST THE DEMONSTRATION DRILLED SHAFT BY THERMAL INTEGRITY PROFILING (TIP) ACCORDING TO ASTM D7949, METHOD BE, BY CROSSHOLE SONIC LOGGING (CSL) ACCORDING TO ASTM D6760; AND BY HIGH-STRAIN DYNAMIC TESTING ACCORDING TO ASTM D4945.

PART 4: MEASUREMENT AND PAYMENT

THE DEPARTMENT WILL MEASURE DEMONSTRATION DRILLED SHAFT BY THE NUMBER OF FEET, MEASURED ALONG THE AXIS OF THE DRILLED SHAFT FROM THE REQUIRED BOTTOM ELEVATION OF THE SHAFT TO THE PROPOSED TOP PLAN ELEVATION.

IN ADDITION TO THE PROVISIONS OF C&MS 524.17, THE DEPARTMENT WILL PAY FOR ACCEPTED QUANTITIES OF DEMONSTRATION DRILLED SHAFT AFTER INSTALLATION OF THE DEMONSTRATION SHAFT AND AFTER BEING PROVIDED WITH WRITTEN TESTING AND EVALUATION RESULTS ACCEPTABLE TO THE ENGINEER.

THE CONTRACT PRICE IS FULL COMPENSATION FOR FURNISHING AND INSTALLING DRILLED SHAFTS IN ACCORDANCE WITH THE ABOVE REQUIREMENTS, INCLUDING MOBILIZATION, SITE ACCESS, AND FINAL REMOVAL OF THE SHAFT TO 36 INCHES BELOW FINAL GRADE.

CUY-90-16.28 (CCG3A)

MODEL: Sheet (2 of 3) PAPER: 17x11 (in.) DATE: 8/11/2022 TIME: 3:21:33 PM USER: Shane.Wells pwc:\mb-us-pw-bentley.com\mb-us-pw-03\Documents\Cleveland_OH101_Projects\ODOT\Drawings\1282382400-Engineering\Structures\WALL_AG_Sheets\82382_AG_VN001.dgn

WALL GENERAL NOTES (2 OF 3)
WALL AG
ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC 08-04-22	
PROJECT ID	
82382	
SUBSET	TOTAL
4	18
SHEET	
TOTAL	
1041	2338

THE DEPARTMENT WILL PAY FOR TESTING AND EVALUATION OF THE ACCEPTED DEMONSTRATION SHAFT SEPARATELY.

THE DEPARTMENT WILL NOT PAY FOR TESTING AND EVALUATION FOR ADDITIONAL DEMONSTRATION DRILLED SHAFTS.

THE DEPARTMENT WILL PAY FOR ACCEPTED QUANTITIES AT THE CONTRACT PRICE AS FOLLOWS: ITEM 524 DRILLED SHAFTS, MISC.: DEMONSTRATION DRILLED SHAFT.

ITEM 894 - THERMAL INTEGRITY PROFILER (T.I.P.) TEST

PERFORM INTEGRITY TESTING ON THE DEMONSTRATION DRILLED SHAFT AT THE REAR ABUTMENT BY THERMAL INTEGRITY PROFILING (TIP). PERFORM TIP TESTING PER ASTM D7949, "STANDARD TEST METHODS FOR THERMAL INTEGRITY PROFILING OF CONCRETE DEEP FOUNDATIONS," METHOD B, AND PER THE PROJECT SPECIAL PROVISIONS.

ITEM 524 - DRILLED SHAFTS, MISC.: CSL TESTING, 48" DIA. SHAFT

PERFORM INTEGRITY TESTING ON THE DEMONSTRATION DRILLED SHAFTS AT THE REAR ABUTMENT BY CROSSHOLE SONIC LOGGING (CSL). PERFORM CSL TESTING PER ASTM D6760, "STANDARD TEST METHOD FOR INTEGRITY TESTING OF CONCRETE DEEP FOUNDATIONS BY ULTRASONIC CROSSHOLE TESTING." AND PER THE PROJECT SPECIAL PROVISIONS.

ITEM 524 - DRILLED SHAFTS, MISC.: HIGH-STRAIN DYNAMIC TESTING OF DRILLED SHAFTS

PERFORM FIELD VERIFICATION OF NOMINAL AXIAL RESISTANCE TESTING ON THE DEMONSTRATION DRILLED SHAFT AT THE REAR ABUTMENT BY HIGH-STRAIN DYNAMIC TESTING. PERFORM HIGH-STRAIN DYNAMIC TESTING PER ASTM D4945, "STANDARD TEST METHOD FOR HIGH-STRAIN DYNAMIC TESTING OF DEEP FOUNDATIONS" AND PER THE PROJECT SPECIAL PROVISIONS.

ITEM 607 - VANDAL PROTECTION FENCE, 8' STRAIGHT, COATED FABRIC, AS PER PLAN:

INSTALL VANDAL PROTECTION FENCE ACCORDING TO STD. CONSTRUCTION DRAWING VPF-1-90 AND C&MS 607, EXCEPT AS MODIFIED BELOW.

POSTS, PLATES, TIE WIRES, CAULK AND ADDITIONAL VISIBLE HARDWARE SHALL BE COLOR BLACK (FEDERAL STD. 595C #17038). FENCE FABRIC SHALL BE BLACK VINYL-COATED, CHAIN LINK STYLE. MOUNT FENCING TO TOP OF RETAINING WALL WITH CAST-IN-PLACE ANCHORS.

ITEM SPECIAL - RETAINING WALL, MISC.: TIMBER LAGGING

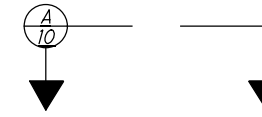
THIS ITEM CONSISTS OF FURNISHING AND INSTALLING UNTREATED HARDWOOD LAGGING TO SERVE AS TEMPORARY LAGGING FOR THE SOLDIER PILE WALL. THE LAGGING SHALL CONSIST OF HARDWOOD TIMBER WITH 3 INCH BY 8 INCH DIMENSIONS AND SHALL BE OF A GRADE AND TYPE WITH AN ALLOWABLE EXTREME FIBER STRESS IN BENDING OF A MINIMUM OF 1 KSI. THE TIMBER MATERIAL SHALL BE DOUGLAS FIR-LARCH DENSE NO. 1, SELECT STRUCTURAL OR DENSE SELECT STRUCTURAL. THE WOOD SHALL BE SEASONED, SOUND, AND FREE FROM DECAY AND INSECT ATTACK, WITH NO LOOSE AND/OR CLUSTER KNOTS. THE ENDS OF THE TIMBER SHALL BE SAWED SQUARE WITH THE AXIS OF THE TIMBER. THE TIMBER MEMBERS SHALL ALSO CONFORM TO C&MS 711.26. PROVIDE CERTIFICATION THAT THE TIMBER CONFORMS TO THE GRADE, SPECIES AND OTHER SPECIFIED REQUIREMENTS.

LAGGING SHALL BE PLACED IN A TOP-DOWN MANNER AS EXCAVATION PROCEEDS DOWNWARD. AT NO TIME SHOULD MORE THAN 3 FEET OF UNSUPPORTED EXCAVATION BE PERMITTED. REDUCE THE UNSUPPORTED HEIGHT AS NECESSARY TO PREVENT CAVING AND SLOUGHING OF THE SOILS BETWEEN THE SOLDIER PILES. PROVIDE 1/4 INCH TO 1/2 INCH WOOD SPACERS TO PROVIDE HORIZONTAL JOINT SPACING BETWEEN THE LAGGING BOARDS TO PERMIT DRAINAGE.

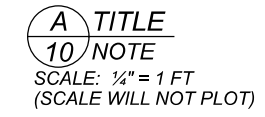
HARDWOOD SHIMMING SHALL BE USED AS NEEDED TO MAINTAIN THE POSITIONING OF THE TIMBER MEMBERS AGAINST THE INTERIOR FLANGE FACE OF THE SOLDIER PILE DURING CONSTRUCTION OF THE RETAINING WALL SYSTEM. NO SHIM SHALL EXTEND BEYOND THE FLANGE OF THE H-PILE MORE THAN 1/4 INCH AND NO SHIM SHALL EXTEND BEYOND THE TOP FACE OF THE TOP LAGGING.

THE DEPARTMENT WILL MEASURE THE TEMPORARY HARDWOOD LAGGING BY THE NUMBER OF SQUARE FEET. PAYMENT SHALL INCLUDE ALL LABOR, MATERIAL AND INCIDENTALS (INCLUDING HARDWOOD SHIMMING AND WOOD SPACERS) NECESSARY TO FURNISH AND INSTALL THE TEMPORARY HARDWOOD LAGGING.

SECTION/DETAIL/VIEW CALLOUTS



(SEE SECTION A ON SHEET 10)



(SECTION A CUT FROM SHEET 10)

PLAN ABBREVIATIONS:

- ABUT. = ABUTMENT
- APPR. = APPROACH
- B = BOTTOM
- BL = BASELINE
- B.F. = BACK FACE
- BM = BENCHMARK
- BOT. OR BTM. = BOTTOM
- CL = CENTERLINE
- C/C = CENTER TO CENTER
- C.I.P. = CAST-IN-PLACE
- C.J. = CONSTRUCTION JOINT
- CLR. = CLEAR
- CMS = CONSTRUCTION AND MATERIAL SPECIFICATIONS
- CONC. = CONCRETE
- CONST. = CONSTRUCTION
- DIA. = DIAMETER
- DIM. = DIMENSION
- DTBD = DISPOSITION TO BE DETERMINED
- DWG. = DRAWING
- EB = EASTBOUND
- E.F. = EACH FACE
- EL. OR ELEV. = ELEVATION
- EQ. = EQUAL
- EST. = ESTIMATED
- EX. = EXISTING
- F.A. = FORWARD ABUTMENT
- F/F = FACE TO FACE
- F.F. = FRONT FACE
- FT. = FOOT OR FEET
- FTG. = FOOTING
- FWD. = FORWARD
- IN. = INCH
- JT. = JOINT
- LT. = LEFT
- MAX. = MAXIMUM
- MIN. = MINIMUM
- MISC. = MISCELLANEOUS
- N = NORTH
- NB = NORTHBOUND
- NO. = NUMBER
- N.P.C.P.P. = NON-PERFORATED CORRUGATED PLASTIC PIPE
- OHWM = ORDINARY HIGH WATER MARK
- O/O = OUT TO OUT
- P.C.P.P. = PERFORATED CORRUGATED PLASTIC PIPE
- P.E.J.F. = PREFORMED EXPANSION JOINT FILLER
- PROP. = PROPOSED
- PSF = POUNDS PER SQUARE FOOT
- R.A. = REAR ABUTMENT
- S = SOUTH
- SB = SOUTHBOUND
- SER. = SERIES
- SHLDR = SHOULDER
- SPA. = SPACE OR SPACES
- STA. = STATION
- STD. = STANDARD
- STR = STRAIGHT
- T = TOP
- T&B = TOP & BOTTOM
- TBR = TO BE REMOVED
- TBRBO = TO BE RELOCATED BY OTHERS
- TEMP. = TEMPORARY
- TYP. = TYPICAL
- U.N.O. = UNLESS NOTED OTHERWISE
- VAR. = VARIES
- WB = WESTBOUND
- WWR = WELDED WIRE REINFORCEMENT

CUY-90-16.28 (CCG3A)

MODEL: Sheet (3 of 3) PAPER SIZE: 17x11 (in.) DATE: 8/11/2022 TIME: 3:21:35 PM USER: Shane.Wells pwc:\mb-us-pw-bentley.com\mb-us-pw-03\Documents\Cleveland_OH101_P\Projects\ODOT\Dist\12182382400-Engineering\Structures\WALL_AG_Sheets\82382_AG_WN001.dgn

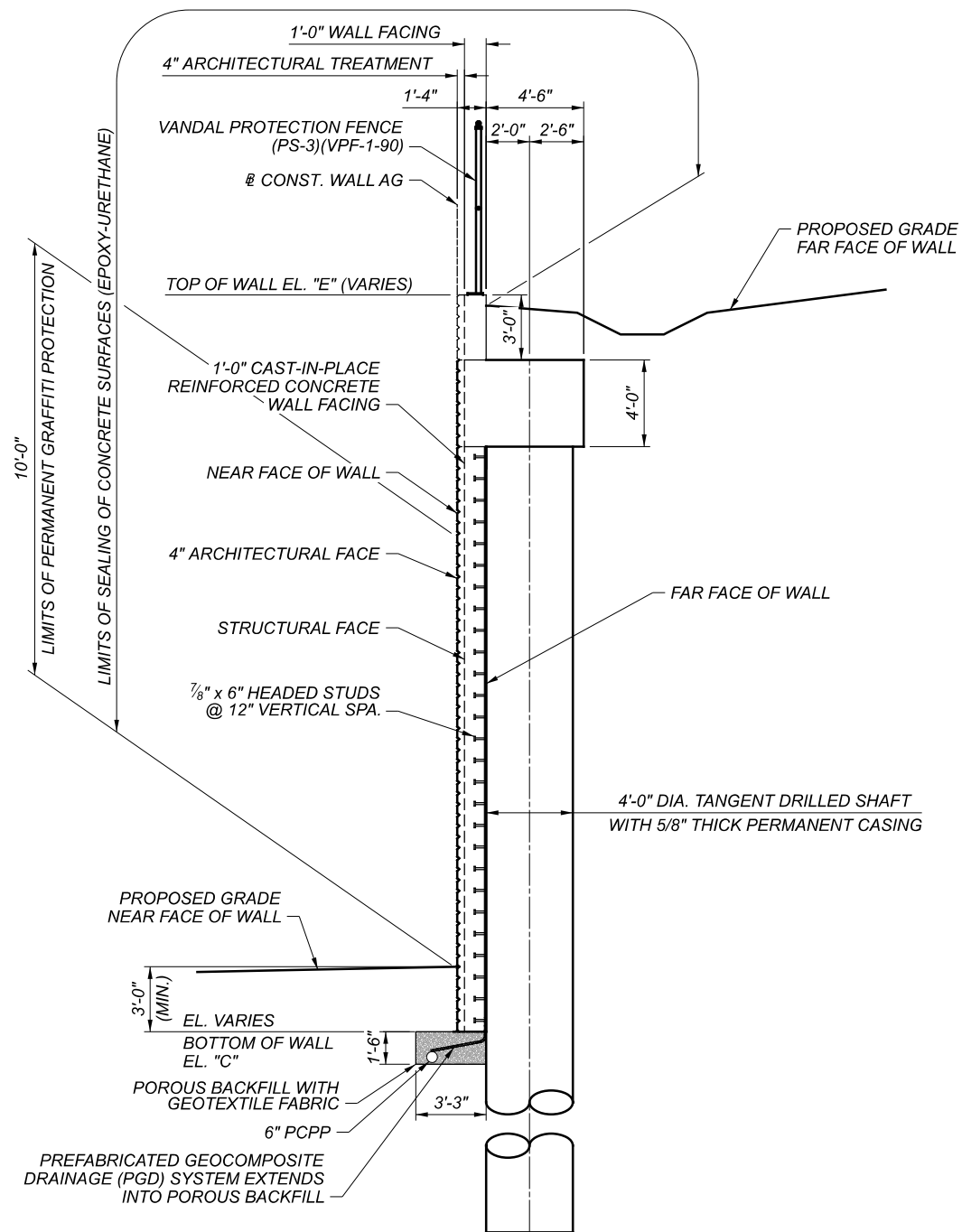
WALL GENERAL NOTES (3 OF 3)
WALL AG
ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
Michael Baker	INTERNATIONAL
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC	08-04-22
PROJECT ID	82382
SUBSET	TOTAL
5	18
SHEET	TOTAL
1041A	2338

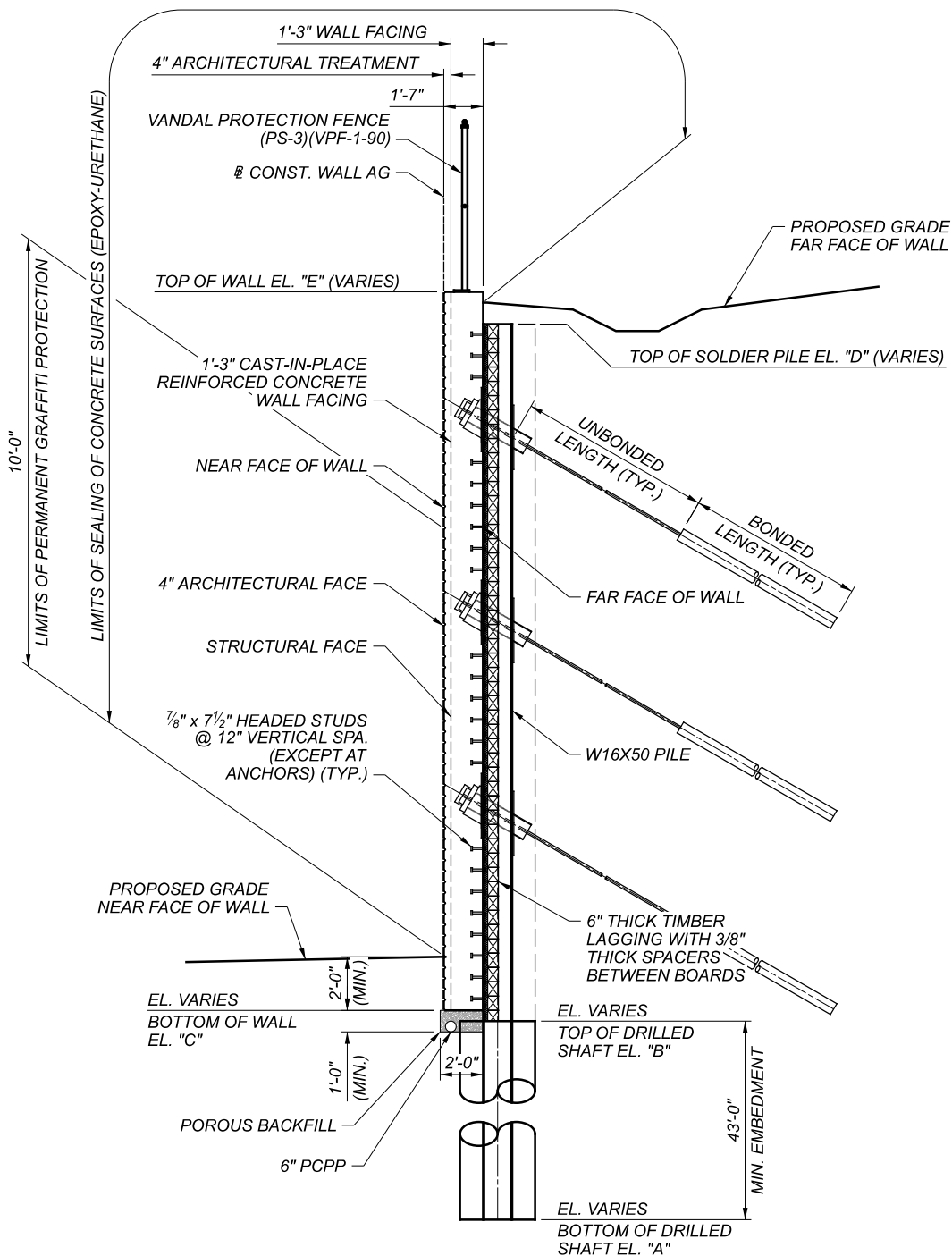
ITEM NO.	EXT.	TOTAL	UNIT	DESCRIPTION	ABUT	PIERS	SUPER	GEN	AS PER PLAN
503	21301	0	LS	UNCLASSIFIED EXCAVATION, AS PER PLAN (FOR ASPL WALL CONSTRUCTION)					3
507	00400	0	FT	STEEL PILES, MISC.: W16x50, FURNISHED					3
509	10000	0	LB	EPOXY COATED REINFORCING STEEL					
511	44113	0	CY	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING, AS PER PLAN					3
511	46013	0	CY	CLASS QC1 CONCRETE WITH QC/QA, RETAINING/WINGWALL NOT INCLUDING FOOTING, AS PER PLAN					3
511	71200	0	SF	CONCRETE, MISC.: ARCHITECTURAL TREATMENT					
512	10001	0	SY	SEALING OF CONCRETE SURFACES, AS PER PLAN (PERMANENT GRAFFITTI PROTECTION)					3
512	10100	0	SY	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)					3
512	33001	0	SY	TYPE 2 WATERPROOFING, AS PER PLAN					3
513	20000	0	EACH	WELDED STUD SHEAR CONNECTORS (7/8" X 7 1/2" ASPL WALL)					
513	20000	0	EACH	WELDED STUD SHEAR CONNECTORS (7/8" X 6" TDS WALL)					
518	20000	0	SY	PREFABRICATED GEOCOMPOSITE DRAIN					4
518	21200	0	CY	POROUS BACKFILL WITH GEOTEXTILE FABRIC					
518	40000	0	FT	6" PERFORATED CORRUGATED PLASTIC PIPE					
518	40010	0	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS					
524	94801	0	FT	DRILLED SHAFTS, 42" DIAMETER, AS PER PLAN					4
524	94802	0	FT	DRILLED SHAFTS, 42" DIAMETER, ABOVE BEDROCK, AS PER PLAN					4
524	94901	0	FT	DRILLED SHAFTS, 48" DIAMETER, AS PER PLAN					4
524	95100	0	EACH	DRILLED SHAFTS, MISC.: DEMONSTRATION DRILLED SHAFT					4
524	95100	0	EACH	DRILLED SHAFTS, MISC.: CSL TESTING, 48" DIA. SHAFT					5
524	95100	0	EACH	DRILLED SHAFTS, MISC.: HIGH-STRAIN DYNAMIC TESTING OF DRILLED SHAFTS					5
524	95100	0	EACH	DRILLED SHAFTS, MISC.: THERMAL INTEGRITY PROFILER (TIP) TEST					5
607	39911	0	FT	VANDAL PROTECTION FENCE, 8' STRAIGHT, COATED FABRIC, AS PER PLAN					5
530	51020	0	SF	SPECIAL - RETAINING WALL, TIMBER LAGGING					5
866	00101	0	EACH	GROUND ANCHOR, AS PER PLAN (109 KIP MAX. TEST LOAD)					12, 13
866	00101	0	EACH	GROUND ANCHOR, AS PER PLAN (133 KIP MAX. TEST LOAD)					12, 13
866	00101	0	EACH	GROUND ANCHOR, AS PER PLAN (149 KIP MAX. TEST LOAD)					12, 13
866	00300	0	LS	INVESTIGATIVE ANCHOR PULLOUT TESTS					
866	00400	0	EACH	PERFORMANCE TEST					

ESTIMATED QUANTITIES
 WALL AG
 ALONG SOUTH SIDE OF I.R. 90

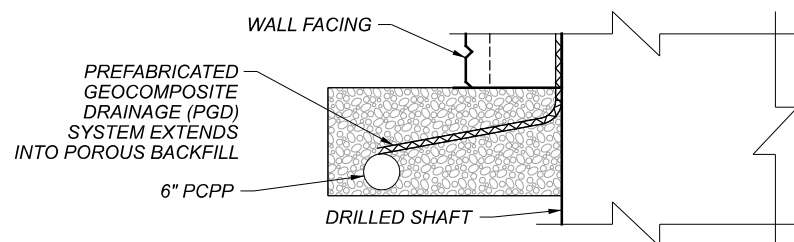
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Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC	08-04-22
PROJECT ID	
	82382
SUBSET	TOTAL
6	18
SHEET	TOTAL
1042	2338



FRICITION TANGENT DRILLED SHAFT WALL TYP. SECTION
 (CAST IN PLACE REINFORCED CONCRETE FACING)
 (REINFORCING STEEL NOT SHOWN FOR CLARITY)



ANCHORED SOLDIER PILE AND LAGGING WALL TYP. SECTION
 (CAST IN PLACE REINFORCED CONCRETE FACING)
 (REINFORCING STEEL NOT SHOWN FOR CLARITY)



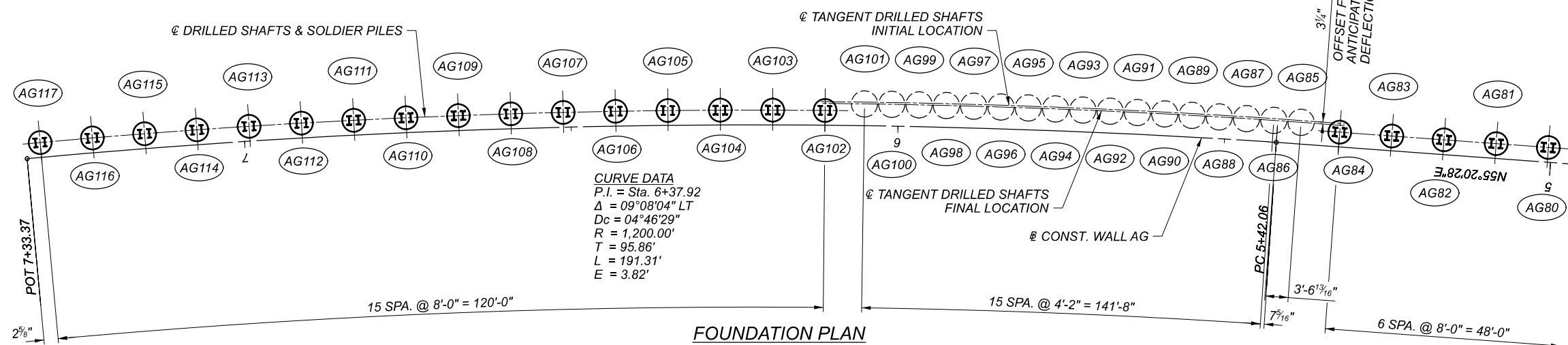
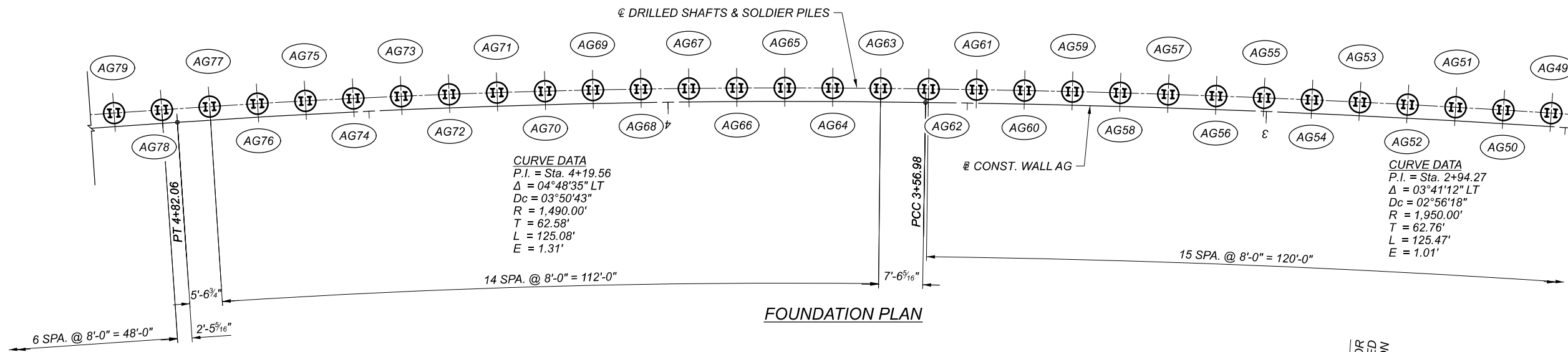
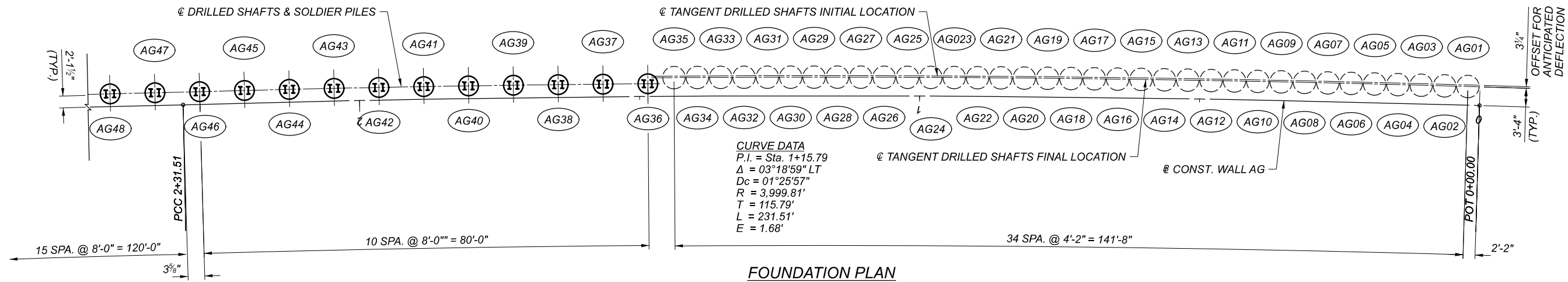
FACING DRAINAGE DETAIL

NOTES

1. FOR THE FOUNDATION PLAN, SEE SHEET 8 / 18
2. FOR THE DRILLED SHAFT TABLE, SEE SHEET 9 / 18
3. FOR THE SOLDIER PILE TABLE, SEE SHEET 10 / 18
4. FOR THE ANCHOR TABLES, SEE SHEETS 11 TO 12 / 18

WALL TYPICAL SECTION
 WALL AG
 ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPC
PROJECT ID	82382
SUBSET	7
TOTAL	18
SHEET	1043
TOTAL	2338



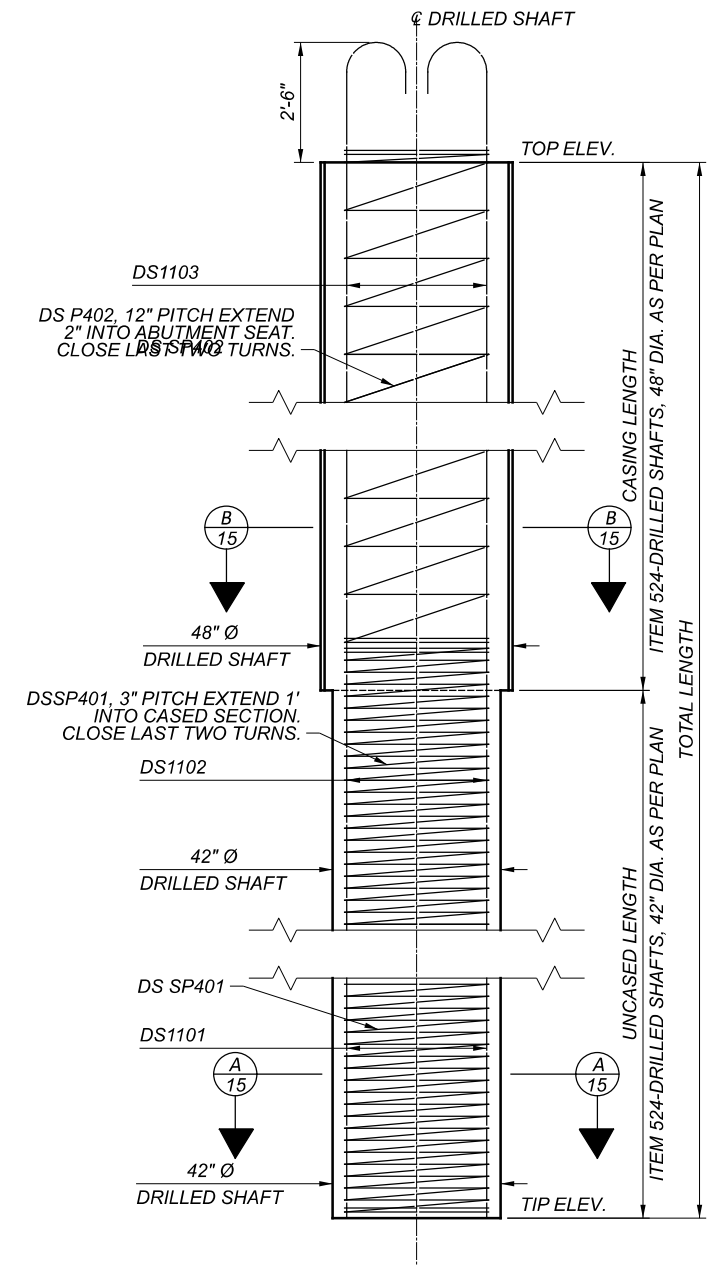
NOTES:

1. FOR THE TANGENT DRILLED SHAFT SCHEDULE TABLES, SEE SHEET 9 / 18
2. FOR THE DRILLED SHAFT & SOLDIER PILE SCHEDULE TABLES, SEE SHEET 10 / 18

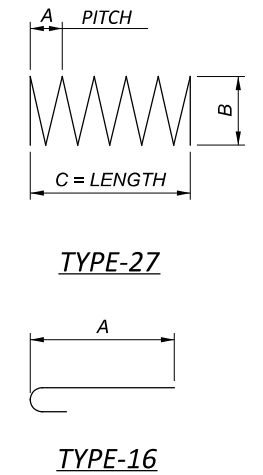
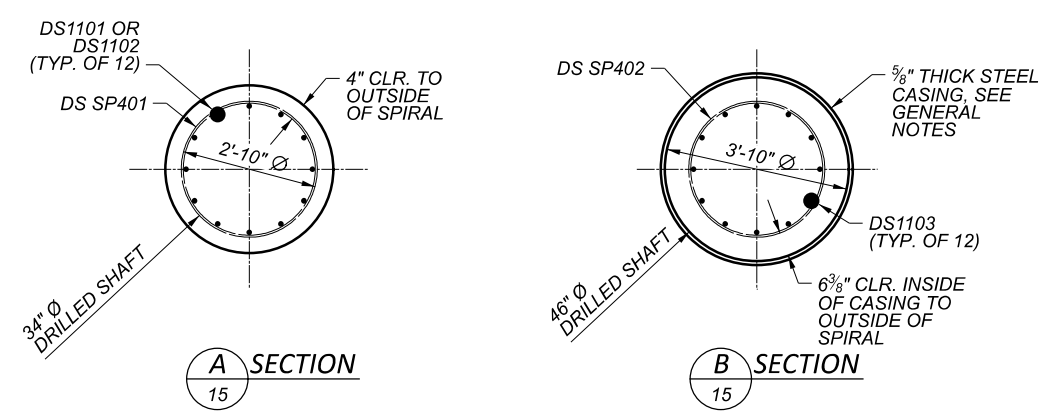
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DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPZ
PROJECT ID	82382
SUBSET	8
TOTAL	18
SHEET	1044
TOTAL	2338

WALL AG TANGENT DRILLED SHAFT SCHEDULE

DESIGNATION	INITIAL NORTHING	INITIAL EASTING	DIAMETER (IN.)	TOP ELEV.	TIP ELEV.	TOTAL LENGTH (FT.)	D.S. BAR MARK	NO. OF D.S. LONG. BARS	SPIRAL MARK	CASING LENGTH (FT.)	CASING THICKNESS (IN.)	DEFLECTED NORTHING	DEFLECTED EASTING
AG01	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG02	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG03	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG04	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG05	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG06	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG07	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG08	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG09	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG10	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG11	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG12	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG13	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG14	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG15	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG16	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG17	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG18	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG19	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG20	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG21	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG22	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG23	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG24	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG25	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG26	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG27	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG28	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG29	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG30	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG31	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG32	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG33	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG34	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG35	0.00	0.00	48	663.00	550.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG85	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG86	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG87	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG88	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG89	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG90	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG91	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG92	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG93	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG94	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG95	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG96	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG97	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG98	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG99	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG100	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00
AG101	0.00	0.00	48	661.00	548.00	113.0	DS1101, DS1102 AND DS1103	3 - S.O. 12	DSDP401 AND DSSP402	80.0	0.625	0.00	0.00



DRILLED SHAFT ELEVATION



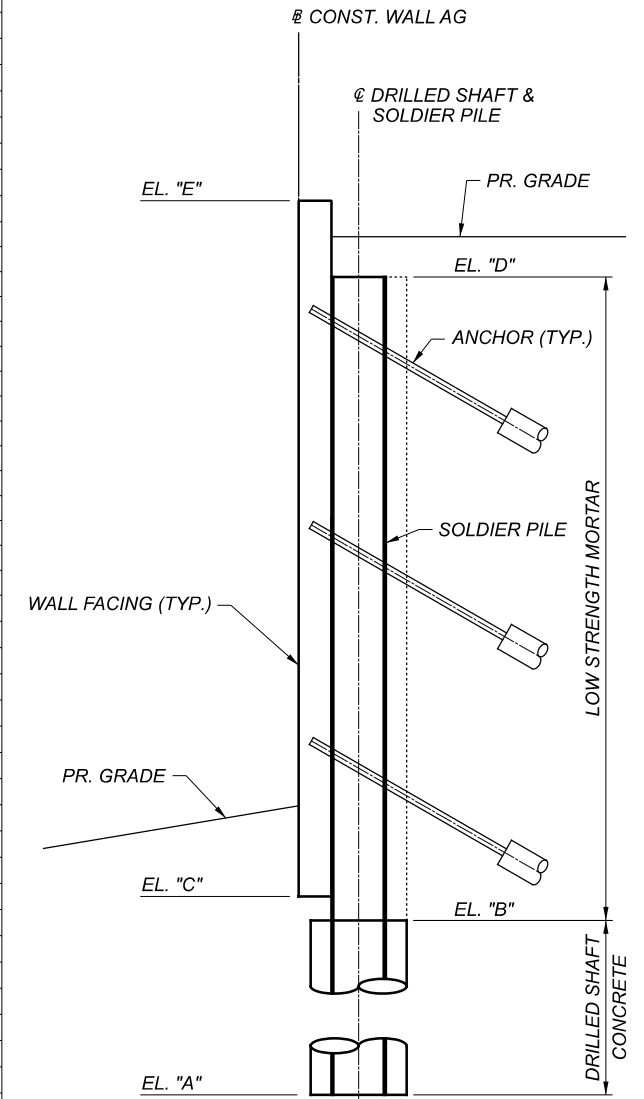
BAR MARK	LENGTH	TYPE	A	B	C
DS1101	40'-0"	STR.			
DS1102	30'-0"	STR.			
DS1103	2'-6"	16			
DS SP401		27	3"		2'-10"
DS SP402		27	12"		2'-10"

- NOTES:
- FOR THE FOUNDATION PLAN, SEE SHEET 8 / 18
 - DRILLED SHAFTS ARE TO BE INSTALLED IN THE INITIAL POSITION INDICATED IN THE FOUNDATION PLAN. REFER TO THE GENERAL NOTES FOR DETAILS.
 - PAYMENT FOR DRILLED SHAFTS INSTALLED IN ACCORDANCE WITH THE PLANS AND SPECIFICATIONS SHALL BE MADE UNDER ITEM 524-DRILLED SHAFTS, 42" DIAMETER, AS PER PLAN AND ITEM 524-DRILLED SHAFTS, 48" DIAMETER, AS PER PLAN. REFER TO GENERAL NOTES.

DRILLED SHAFT TABLE
 WALL AG
 ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	Michael Baker INTERNATIONAL
DESIGNER/CHECKER	SSW / GZ
REVIEWER	LPC 08-04-22
PROJECT ID	82382
SUBSET	9 / 18
SHEET	1045 / 2338

WALL AG DRILLED SHAFT & SOLDIER PILE SCHEDULE													
DESIGNATION	STATION BASELINE WALL AG	CENTERLINE OFFSET FROM BASELINE WALL AG (FT.)	DIAMETER (IN.)	SHEAR STUDS (YES/NO)	BOTTOM OF DRILLED SHAFT EL. "A"	TOP OF SHAFT CONCRETE EL. "B"	CONCRETE SHAFT LENGTH (FT.)	BOTTOM OF WALL EL. "C"	TOP OF SOLDIER PILE E. "D"	TOP OF WALL EL. "E"	ESTIMATED LENGTH OF SOLDIER PILE (FT.)	HEIGHT OF FACING (FT.)	SOLDIER PILE SIZE
AG36	01+48.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.4	669.9	73.0	30.6	W16x50
AG37	01+56.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.4	669.9	73.0	30.6	W16x50
AG38	01+64.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.3	669.8	73.0	30.6	W16x50
AG39	01+72.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.3	669.8	73.0	30.5	W16x50
AG40	01+80.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.3	669.8	73.0	30.5	W16x50
AG41	01+88.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.2	669.7	73.0	30.5	W16x50
AG42	01+96.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.2	669.7	73.0	30.4	W16x50
AG43	02+04.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.2	669.7	73.0	30.4	W16x50
AG44	02+12.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.1	669.6	73.0	30.4	W16x50
AG45	02+20.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.1	669.6	73.0	30.3	W16x50
AG46	02+28.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.1	669.6	73.0	30.3	W16x50
AG47	02+36.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.0	669.5	73.0	30.3	W16x50
AG48	02+44.50	2.26 RT	42	YES	595.8	638.8	43.0	639.3	668.0	669.5	73.0	30.2	W16x50
AG49	02+52.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	668.0	669.5	73.0	30.9	W16x50
AG50	02+60.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.9	669.4	73.0	30.8	W16x50
AG51	02+68.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.9	669.4	73.0	30.8	W16x50
AG52	02+76.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.9	669.4	73.0	30.8	W16x50
AG53	02+84.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.8	669.3	73.0	30.7	W16x50
AG54	02+92.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.8	669.3	73.0	30.7	W16x50
AG55	03+00.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.8	669.3	73.0	30.7	W16x50
AG56	03+08.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.7	669.2	73.0	30.6	W16x50
AG57	03+16.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.7	669.2	73.0	30.6	W16x50
AG58	03+24.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.7	669.2	73.0	30.6	W16x50
AG59	03+32.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.6	669.1	73.0	30.5	W16x50
AG60	03+40.50	2.26 RT	42	YES	595.1	638.1	43.0	638.6	667.6	669.1	73.0	30.5	W16x50
AG61	03+48.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.6	669.1	73.0	30.8	W16x50
AG62	03+56.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.5	669.0	73.0	30.8	W16x50
AG63	03+64.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.5	669.0	73.0	30.7	W16x50
AG64	03+72.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.5	669.0	73.0	30.7	W16x50
AG65	03+80.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.4	668.9	73.0	30.7	W16x50
AG66	03+88.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.4	668.9	73.0	30.6	W16x50
AG67	03+96.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.4	668.9	73.0	30.6	W16x50
AG68	04+04.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.3	668.8	73.0	30.6	W16x50
AG69	04+12.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.3	668.8	73.0	30.5	W16x50
AG70	04+20.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.3	668.8	73.0	30.5	W16x50
AG71	04+28.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.2	668.7	73.0	30.5	W16x50
AG72	04+36.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.2	668.7	73.0	30.4	W16x50
AG73	04+44.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.2	668.7	73.0	30.4	W16x50
AG74	04+52.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.1	668.6	73.0	30.4	W16x50
AG75	04+60.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.1	668.6	73.0	30.3	W16x50
AG76	04+68.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.1	668.6	73.0	30.3	W16x50
AG77	04+76.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.0	668.5	73.0	30.3	W16x50
AG78	04+84.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.0	668.5	73.0	30.2	W16x50
AG79	04+92.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	667.0	668.5	73.0	30.2	W16x50
AG80	05+00.50	2.26 RT	42	YES	594.8	637.8	43.0	638.3	666.9	668.4	73.0	30.2	W16x50
AG81	05+08.50	2.26 RT	42	YES	594.1	637.1	43.0	637.6	666.9	668.4	73.0	30.8	W16x50
AG82	05+16.50	2.26 RT	42	YES	594.1	637.1	43.0	637.6	666.9	668.4	73.0	30.8	W16x50
AG83	05+24.50	2.26 RT	42	YES	594.1	637.1	43.0	637.6	666.8	668.3	73.0	30.8	W16x50
AG84	05+32.50	2.26 RT	42	YES	594.1	637.1	43.0	637.6	666.8	668.3	73.0	30.7	W16x50
AG102	06+11.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.5	668.0	75.0	32.1	W16x50
AG103	06+19.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.4	667.9	75.0	32.0	W16x50
AG104	06+27.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.4	667.9	74.0	32.0	W16x50
AG105	06+35.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.4	667.9	74.0	32.0	W16x50
AG106	06+43.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.3	667.8	74.0	31.9	W16x50
AG107	06+51.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.3	667.8	74.0	31.9	W16x50
AG108	06+59.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.3	667.8	74.0	31.9	W16x50
AG109	06+67.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.2	667.7	74.0	31.8	W16x50
AG110	06+75.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.2	667.7	74.0	31.8	W16x50
AG111	06+83.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.2	667.7	74.0	31.8	W16x50
AG112	06+91.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.1	667.6	74.0	31.7	W16x50
AG113	06+99.16	2.26 RT	42	YES	592.4	635.4	43.0	635.9	666.1	667.6	74.0	31.7	W16x50
AG114	07+07.16	2.26 RT	42	YES	591.4	634.4	43.0	634.9	666.1	667.6	75.0	32.7	W16x50
AG115	07+15.16	2.26 RT	42	YES	591.4	634.4	43.0	634.9	666.0	667.5	75.0	32.6	W16x50
AG116	07+23.16	2.26 RT	42	YES	591.4	634.4	43.0	634.9	666.0	667.5	75.0	32.6	W16x50
AG117	07+31.16	2.26 RT	42	YES	591.4	634.4	43.0	634.9	666.0	667.5	75.0	32.6	W16x50



NOTES:

1. FOR THE FOUNDATION PLAN, SEE SHEET 8 / 18
2. FOR THE ANCHOR TABLES, SEE SHEETS 11 TO 12 / 18

SOLDIER PILE TABLE
WALL AG
ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
LPC	GZ
REVIEWER	
CDC 08-04-22	
PROJECT ID	82382
SUBSET	TOTAL
10	18
SHEET	TOTAL
1045A	2338

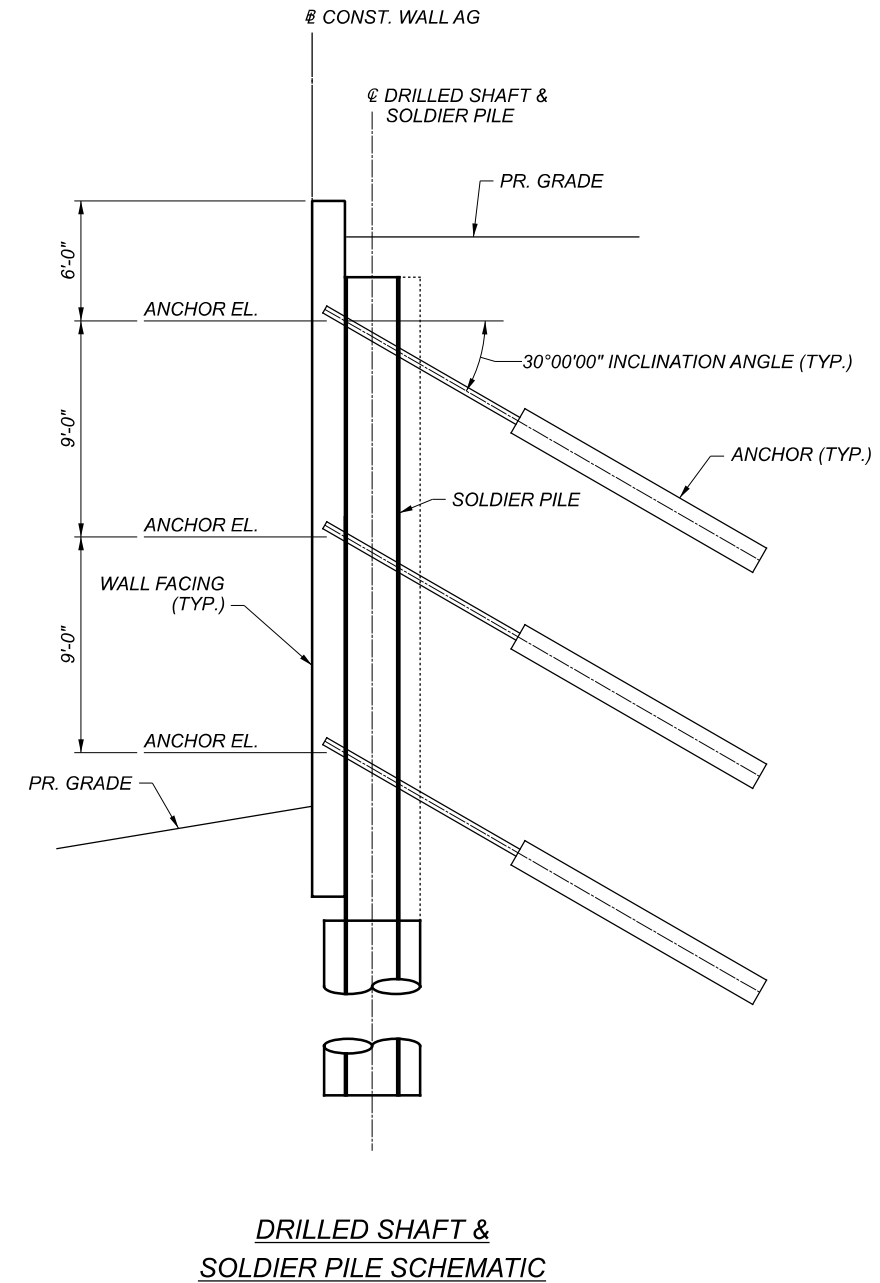
WALL AG ANCHOR TABLE							
ANCHOR DESIGNATION	PILE DESIGNATION	STATION BASELINE WALL AG	ELEVATION	INCLINATION (DEG)	MINIMUM UNBONDED LENGTH (FT)	MAXIMUM TEST LOAD (KIP)	LOCK-OFF LOAD (KIP)
1	AG36	01+48.50	663.89	30	15	133.0	99.0
2	AG36	01+48.50	654.89	30	15	109.0	80.0
3	AG36	01+48.50	645.89	30	15	149.0	99.0
4	AG37	01+56.50	663.86	30	15	133.0	99.0
5	AG37	01+56.50	654.86	30	15	109.0	80.0
6	AG37	01+56.50	645.86	30	15	149.0	99.0
7	AG38	01+64.50	663.83	30	15	133.0	99.0
8	AG38	01+64.50	654.83	30	15	109.0	80.0
9	AG38	01+64.50	645.83	30	15	149.0	99.0
10	AG39	01+72.50	663.79	30	15	133.0	99.0
11	AG39	01+72.50	654.79	30	15	109.0	80.0
12	AG39	01+72.50	645.79	30	15	149.0	99.0
13	AG40	01+80.50	663.76	30	15	133.0	99.0
14	AG40	01+80.50	654.76	30	15	109.0	80.0
15	AG40	01+80.50	645.76	30	15	149.0	99.0
16	AG41	01+88.50	663.73	30	15	133.0	99.0
17	AG41	01+88.50	654.73	30	15	109.0	80.0
18	AG41	01+88.50	645.73	30	15	149.0	99.0
19	AG42	01+96.50	663.69	30	15	133.0	99.0
20	AG42	01+96.50	654.69	30	15	109.0	80.0
21	AG42	01+96.50	645.69	30	15	149.0	99.0
22	AG43	02+04.50	663.66	30	15	133.0	99.0
23	AG43	02+04.50	654.66	30	15	109.0	80.0
24	AG43	02+04.50	645.66	30	15	149.0	99.0
25	AG44	02+12.50	663.62	30	15	133.0	99.0
26	AG44	02+12.50	654.62	30	15	109.0	80.0
27	AG44	02+12.50	645.62	30	15	149.0	99.0
28	AG45	02+20.50	663.59	30	15	133.0	99.0
29	AG45	02+20.50	654.59	30	15	109.0	80.0
30	AG45	02+20.50	645.59	30	15	149.0	99.0
31	AG46	02+28.50	663.56	30	15	133.0	99.0
32	AG46	02+28.50	654.56	30	15	109.0	80.0
33	AG46	02+28.50	645.56	30	15	149.0	99.0
34	AG47	02+36.50	663.52	30	15	133.0	99.0
35	AG47	02+36.50	654.52	30	15	109.0	80.0
36	AG47	02+36.50	645.52	30	15	149.0	99.0
37	AG48	02+44.50	663.49	30	15	133.0	99.0
38	AG48	02+44.50	654.49	30	15	109.0	80.0
39	AG48	02+44.50	645.49	30	15	149.0	99.0
40	AG49	02+52.50	663.46	30	15	133.0	99.0
41	AG49	02+52.50	654.46	30	15	109.0	80.0
42	AG49	02+52.50	645.46	30	15	149.0	99.0
43	AG50	02+60.50	663.42	30	15	133.0	99.0
44	AG50	02+60.50	654.42	30	15	109.0	80.0
45	AG50	02+60.50	645.42	30	15	149.0	99.0
46	AG51	02+68.50	663.39	30	15	133.0	99.0
47	AG51	02+68.50	654.39	30	15	109.0	80.0
48	AG51	02+68.50	645.39	30	15	149.0	99.0
49	AG52	02+76.50	663.36	30	15	133.0	99.0
50	AG52	02+76.50	654.36	30	15	109.0	80.0
51	AG52	02+76.50	645.36	30	15	149.0	99.0
52	AG53	02+84.50	663.32	30	15	133.0	99.0
53	AG53	02+84.50	654.32	30	15	109.0	80.0
54	AG53	02+84.50	645.32	30	15	149.0	99.0
55	AG54	02+92.50	663.29	30	15	133.0	99.0
56	AG54	02+92.50	654.29	30	15	109.0	80.0
57	AG54	02+92.50	645.29	30	15	149.0	99.0
58	AG55	03+00.50	663.26	30	15	133.0	99.0
59	AG55	03+00.50	654.26	30	15	109.0	80.0
60	AG55	03+00.50	645.26	30	15	149.0	99.0
61	AG56	03+08.50	663.22	30	15	133.0	99.0
62	AG56	03+08.50	654.22	30	15	109.0	80.0
63	AG56	03+08.50	645.22	30	15	149.0	99.0
64	AG57	03+16.50	663.19	30	15	133.0	99.0
65	AG57	03+16.50	654.19	30	15	109.0	80.0
66	AG57	03+16.50	645.19	30	15	149.0	99.0

WALL AG ANCHOR TABLE							
ANCHOR DESIGNATION	PILE DESIGNATION	STATION BASELINE WALL AG	ELEVATION	INCLINATION (DEG)	MINIMUM UNBONDED LENGTH (FT)	MAXIMUM TEST LOAD (KIP)	LOCK-OFF LOAD (KIP)
67	AG58	03+24.50	663.16	30	15	133.0	99.0
68	AG58	03+24.50	654.16	30	15	109.0	80.0
69	AG58	03+24.50	645.16	30	15	149.0	99.0
70	AG59	03+32.50	663.12	30	15	133.0	99.0
71	AG59	03+32.50	654.12	30	15	109.0	80.0
72	AG59	03+32.50	645.12	30	15	149.0	99.0
73	AG60	03+40.50	663.09	30	15	133.0	99.0
74	AG60	03+40.50	654.09	30	15	109.0	80.0
75	AG60	03+40.50	645.09	30	15	149.0	99.0
76	AG61	03+48.50	663.06	30	15	133.0	99.0
77	AG61	03+48.50	654.06	30	15	109.0	80.0
78	AG61	03+48.50	645.06	30	15	149.0	99.0
79	AG62	03+56.50	663.02	30	15	133.0	99.0
80	AG62	03+56.50	654.02	30	15	109.0	80.0
81	AG62	03+56.50	645.02	30	15	149.0	99.0
82	AG63	03+64.50	662.99	30	15	133.0	99.0
83	AG63	03+64.50	653.99	30	15	109.0	80.0
84	AG63	03+64.50	644.99	30	15	149.0	99.0
85	AG64	03+72.50	662.96	30	15	133.0	99.0
86	AG64	03+72.50	653.96	30	15	109.0	80.0
87	AG64	03+72.50	644.96	30	15	149.0	99.0
88	AG65	03+80.50	662.92	30	15	133.0	99.0
89	AG65	03+80.50	653.92	30	15	109.0	80.0
90	AG65	03+80.50	644.92	30	15	149.0	99.0
91	AG66	03+88.50	662.89	30	15	133.0	99.0
92	AG66	03+88.50	653.89	30	15	109.0	80.0
93	AG66	03+88.50	644.89	30	15	149.0	99.0
94	AG67	03+96.50	662.85	30	15	133.0	99.0
95	AG67	03+96.50	653.85	30	15	109.0	80.0
96	AG67	03+96.50	644.85	30	15	149.0	99.0
97	AG68	04+04.50	662.82	30	15	133.0	99.0
98	AG68	04+04.50	653.82	30	15	109.0	80.0
99	AG68	04+04.50	644.82	30	15	149.0	99.0
100	AG69	04+12.50	662.79	30	15	133.0	99.0
101	AG69	04+12.50	653.79	30	15	109.0	80.0
102	AG69	04+12.50	644.79	30	15	149.0	99.0
103	AG70	04+20.50	662.75	30	15	133.0	99.0
104	AG70	04+20.50	653.75	30	15	109.0	80.0
105	AG70	04+20.50	644.75	30	15	149.0	99.0
106	AG71	04+28.50	662.72	30	15	133.0	99.0
107	AG71	04+28.50	653.72	30	15	109.0	80.0
108	AG71	04+28.50	644.72	30	15	149.0	99.0
109	AG72	04+36.50	662.69	30	15	133.0	99.0
110	AG72	04+36.50	653.69	30	15	109.0	80.0
111	AG72	04+36.50	644.69	30	15	149.0	99.0
112	AG73	04+44.50	662.65	30	15	133.0	99.0
113	AG73	04+44.50	653.65	30	15	109.0	80.0
114	AG73	04+44.50	644.65	30	15	149.0	99.0
115	AG74	04+52.50	662.62	30	15	133.0	99.0
116	AG74	04+52.50	653.62	30	15	109.0	80.0
117	AG74	04+52.50	644.62	30	15	149.0	99.0
118	AG75	04+60.50	662.59	30	15	133.0	99.0
119	AG75	04+60.50	653.59	30	15	109.0	80.0
120	AG75	04+60.50	644.59	30	15	149.0	99.0
121	AG76	04+68.50	662.55	30	15	133.0	99.0
122	AG76	04+68.50	653.55	30	15	109.0	80.0
123	AG76	04+68.50	644.55	30	15	149.0	99.0
124	AG77	04+76.50	662.52	30	15	133.0	99.0
125	AG77	04+76.50	653.52	30	15	109.0	80.0
126	AG77	04+76.50	644.52	30	15	149.0	99.0
127	AG78	04+84.50	662.49	30	15	133.0	99.0
128	AG78	04+84.50	653.49	30	15	109.0	80.0
129	AG78	04+84.50	644.49	30	15	149.0	99.0
130	AG79	04+92.50	662.45	30	15	133.0	99.0
131	AG79	04+92.50	653.45	30	15	109.0	80.0
132	AG79	04+92.50	644.45	30	15	149.0	99.0

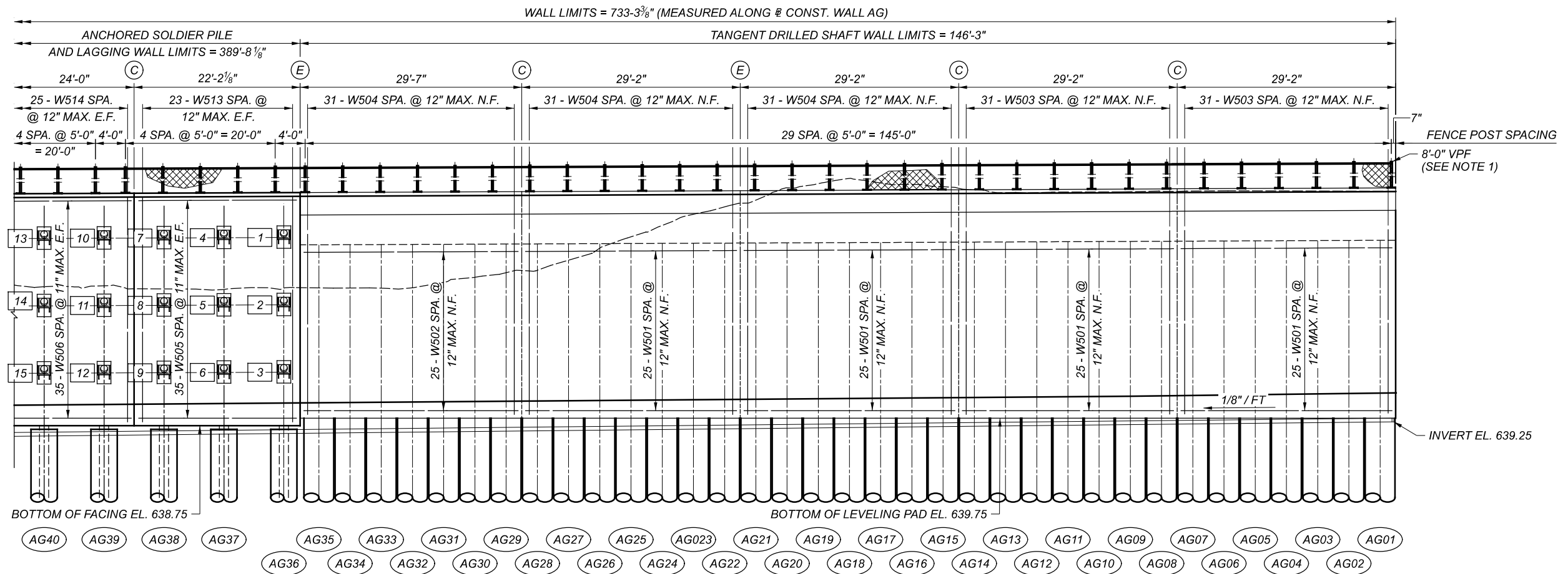
ANCHOR TABLE (1 OF 2)
 WALL AG
 ALONG SOUTH SIDE OF I.R. 90

SFN	N/A
DESIGN AGENCY	
Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC	08-04-22
PROJECT ID	82382
SUBSET	TOTAL
11	18
SHEET	TOTAL
1045B	2338

WALL AG ANCHOR TABLE							
ANCHOR DESIGNATION	PILE DESIGNATION	STATION BASELINE WALL AG	ELEVATION	INCLINATION (DEG)	MINIMUM UNBONDED LENGTH (FT)	MAXIMUM TEST LOAD (KIP)	LOCK-OFF LOAD (KIP)
133	AG80	05+00.50	662.42	30	15	133.0	99.0
134	AG80	05+00.50	653.42	30	15	109.0	80.0
135	AG80	05+00.50	644.42	30	15	149.0	99.0
136	AG81	05+08.50	662.39	30	15	133.0	99.0
137	AG81	05+08.50	653.39	30	15	109.0	80.0
138	AG81	05+08.50	644.39	30	15	149.0	99.0
139	AG82	05+16.50	662.35	30	15	133.0	99.0
140	AG82	05+16.50	653.35	30	15	109.0	80.0
141	AG82	05+16.50	644.35	30	15	149.0	99.0
142	AG83	05+24.50	662.32	30	15	133.0	99.0
143	AG83	05+24.50	653.32	30	15	109.0	80.0
144	AG83	05+24.50	644.32	30	15	149.0	99.0
145	AG84	05+32.50	662.29	30	15	133.0	99.0
146	AG84	05+32.50	653.29	30	15	109.0	80.0
147	AG84	05+32.50	644.29	30	15	149.0	99.0
148	AG102	06+11.16	661.96	30	15	133.0	99.0
149	AG102	06+11.16	652.96	30	15	109.0	80.0
150	AG102	06+11.16	643.96	30	15	149.0	99.0
151	AG103	06+19.16	661.93	30	15	133.0	99.0
152	AG103	06+19.16	652.93	30	15	109.0	80.0
153	AG103	06+19.16	643.93	30	15	149.0	99.0
154	AG104	06+27.16	661.89	30	15	133.0	99.0
155	AG104	06+27.16	652.89	30	15	109.0	80.0
156	AG104	06+27.16	643.89	30	15	149.0	99.0
157	AG105	06+35.16	661.86	30	15	133.0	99.0
158	AG105	06+35.16	652.86	30	15	109.0	80.0
159	AG105	06+35.16	643.86	30	15	149.0	99.0
160	AG106	06+43.16	661.83	30	15	133.0	99.0
161	AG106	06+43.16	652.83	30	15	109.0	80.0
162	AG106	06+43.16	643.83	30	15	149.0	99.0
163	AG107	06+51.16	661.79	30	15	133.0	99.0
164	AG107	06+51.16	652.79	30	15	109.0	80.0
165	AG107	06+51.16	643.79	30	15	149.0	99.0
166	AG108	06+59.16	661.76	30	15	133.0	99.0
167	AG108	06+59.16	652.76	30	15	109.0	80.0
168	AG108	06+59.16	643.76	30	15	149.0	99.0
169	AG109	06+67.16	661.73	30	15	133.0	99.0
170	AG109	06+67.16	652.73	30	15	109.0	80.0
171	AG109	06+67.16	643.73	30	15	149.0	99.0
172	AG110	06+75.16	661.69	30	15	133.0	99.0
173	AG110	06+75.16	652.69	30	15	109.0	80.0
174	AG110	06+75.16	643.69	30	15	149.0	99.0
175	AG111	06+83.16	661.66	30	15	133.0	99.0
176	AG111	06+83.16	652.66	30	15	109.0	80.0
177	AG111	06+83.16	643.66	30	15	149.0	99.0
178	AG112	06+91.16	661.62	30	15	133.0	99.0
179	AG112	06+91.16	652.62	30	15	109.0	80.0
180	AG112	06+91.16	643.62	30	15	149.0	99.0
181	AG113	06+99.16	661.59	30	15	133.0	99.0
182	AG113	06+99.16	652.59	30	15	109.0	80.0
183	AG113	06+99.16	643.59	30	15	149.0	99.0
184	AG114	07+07.16	661.56	30	15	133.0	99.0
185	AG114	07+07.16	652.56	30	15	109.0	80.0
186	AG114	07+07.16	643.56	30	15	149.0	99.0
187	AG115	07+15.16	661.52	30	15	133.0	99.0
188	AG115	07+15.16	652.52	30	15	109.0	80.0
189	AG115	07+15.16	643.52	30	15	149.0	99.0
190	AG116	07+23.16	661.5	30	15	133.0	99.0
191	AG116	07+23.16	652.5	30	15	109.0	80.0
192	AG116	07+23.16	643.5	30	15	149.0	99.0
193	AG117	07+31.16	661.51	30	15	133.0	99.0
194	AG117	07+31.16	652.51	30	15	109.0	80.0
195	AG117	07+31.16	643.51	30	15	149.0	99.0



SFN	N/A
DESIGN AGENCY	
Michael Baker INTERNATIONAL	
DESIGNER	CHECKER
SSW	GZ
REVIEWER	
LPC 08-04-22	
PROJECT ID	
82382	
SUBSET	TOTAL
12	18
SHEET	TOTAL
1045C	2338

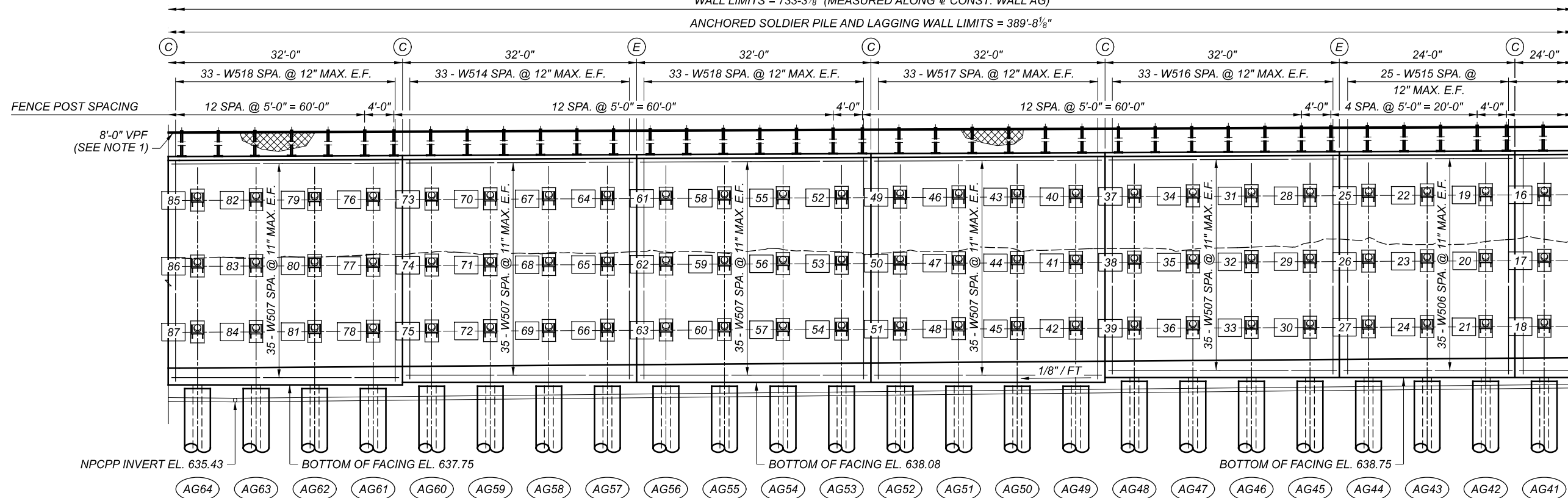


ELEVATION

(DIMENSIONS GIVEN ALONG # CONST. WALL AG)

WALL LIMITS = 733'-3 3/8" (MEASURED ALONG # CONST. WALL AG)

ANCHORED SOLDIER PILE AND LAGGING WALL LIMITS = 389'-8 1/8"



ELEVATION

(DIMENSIONS GIVEN ALONG # CONST. WALL AG)

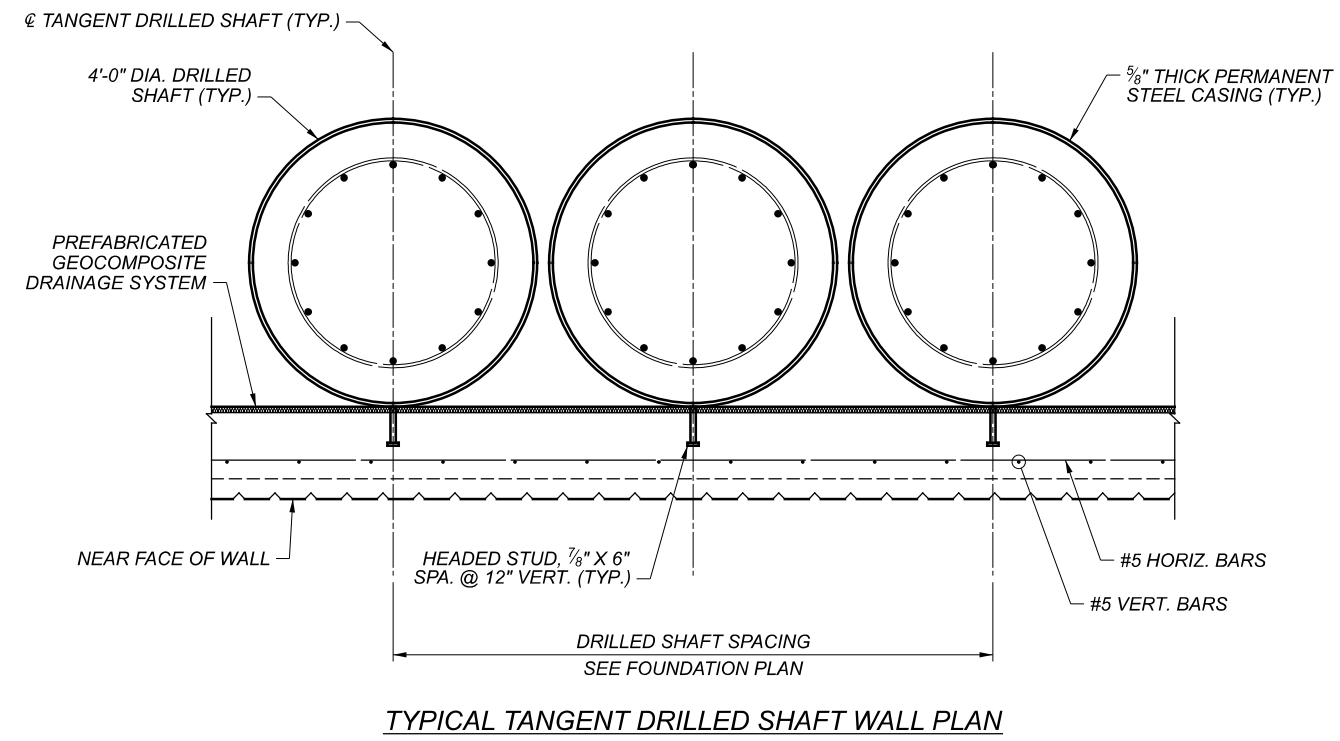
NOTES:

- SEE ODOT STANDARD DRAWING VPF-1-90 FOR FENCE DETAILS.
- FENCE POSTS SHALL BE INSTALLED PLUMB.

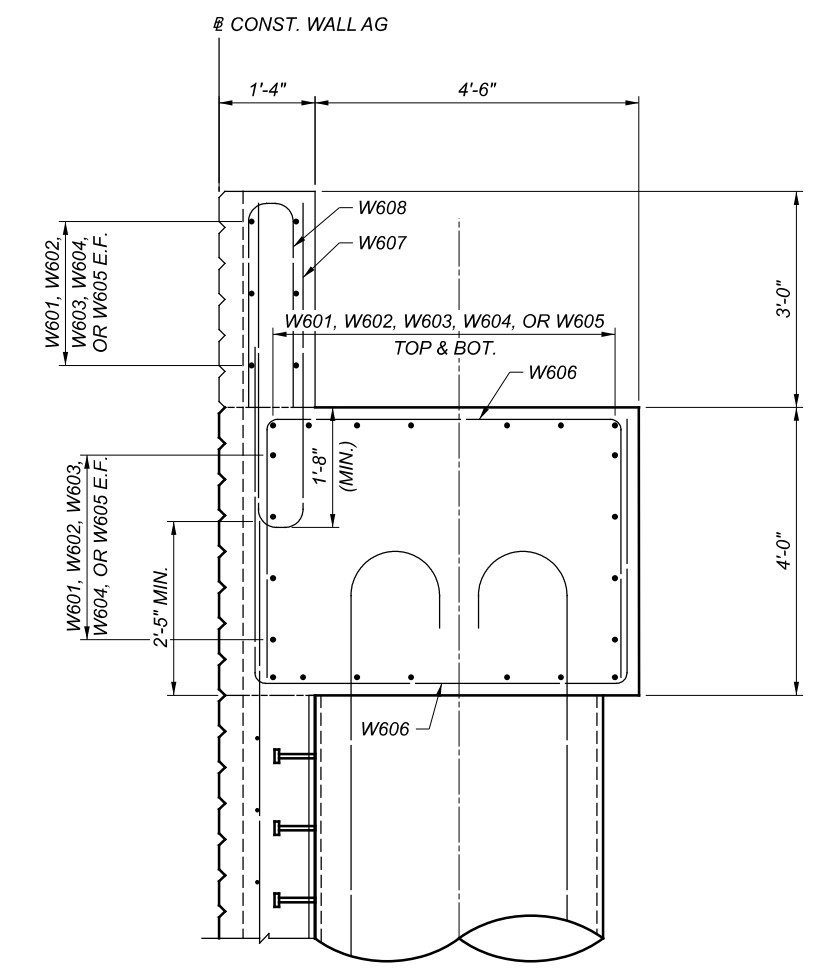
LEGEND:

- # - DENOTES SOLDIER PILE NUMBER
- # - DENOTES ANCHOR NUMBER

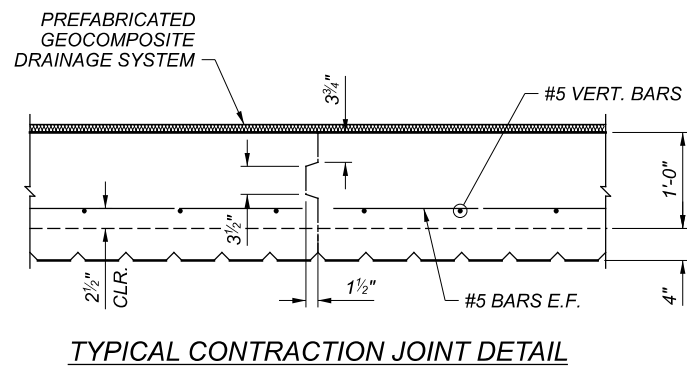
SFN	N/A
DESIGN AGENCY	
DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPC 08-04-22
PROJECT ID	82382
SUBSET	13
TOTAL	18
SHEET	1046
TOTAL	2338



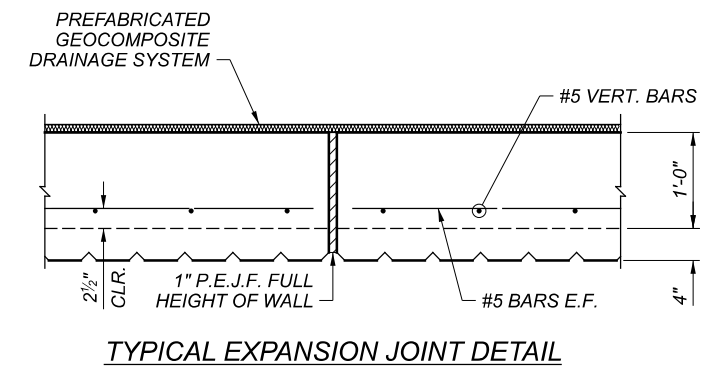
TYPICAL TANGENT DRILLED SHAFT WALL PLAN



TYPICAL TANGENT DRILLED SHAFT CAP SECTION

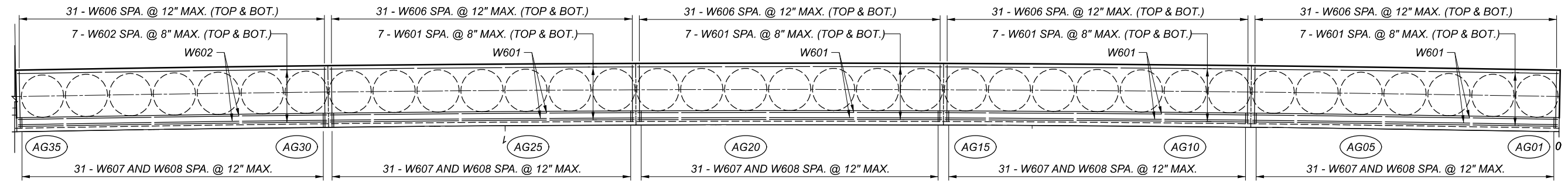


TYPICAL CONTRACTION JOINT DETAIL

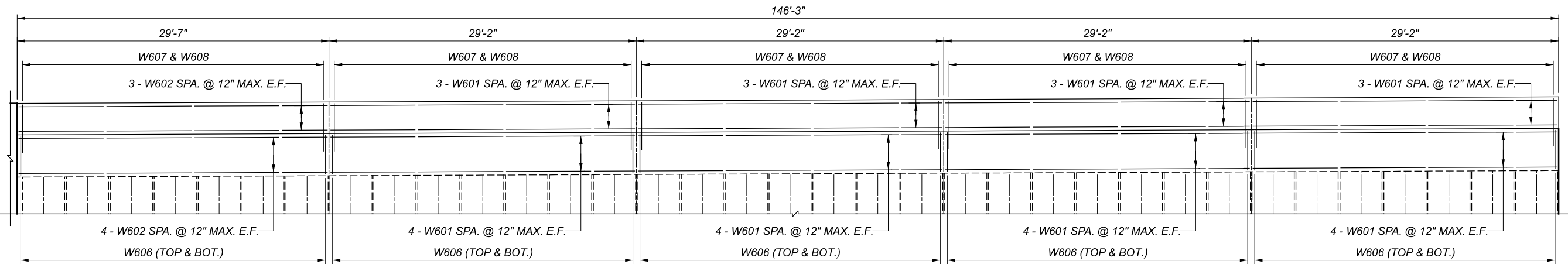


TYPICAL EXPANSION JOINT DETAIL

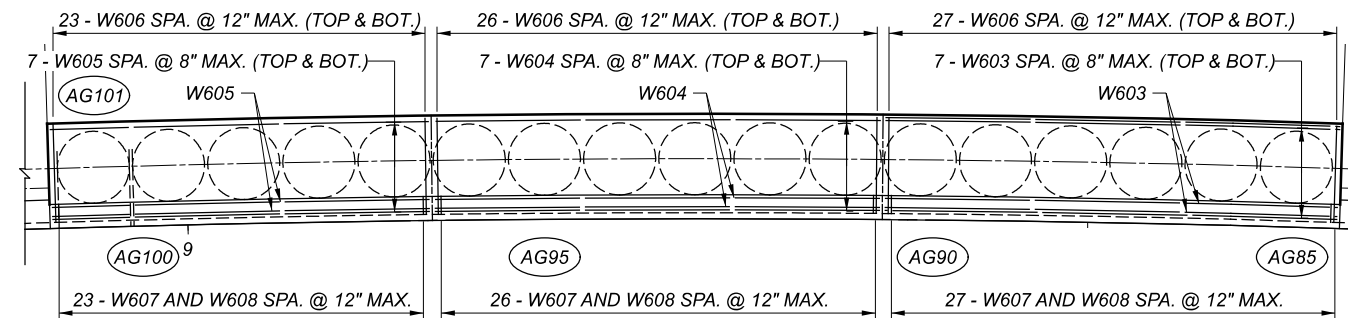
SFN	N/A
DESIGN AGENCY	
DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPC
PROJECT ID	82382
SUBSET	15
TOTAL	18
SHEET	1047
TOTAL	2338



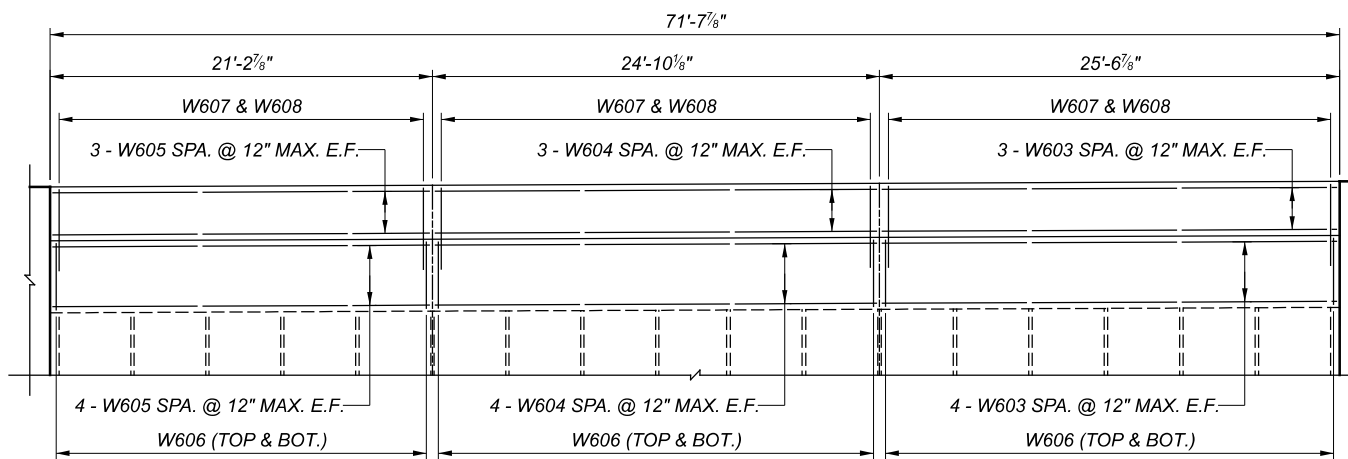
TANGENT DRILLED SHAFT CAP PLAN



TANGENT DRILLED SHAFT CAP ELEVATION
 (DIMENSIONS GIVEN ALONG \perp CONST. WALL AG)



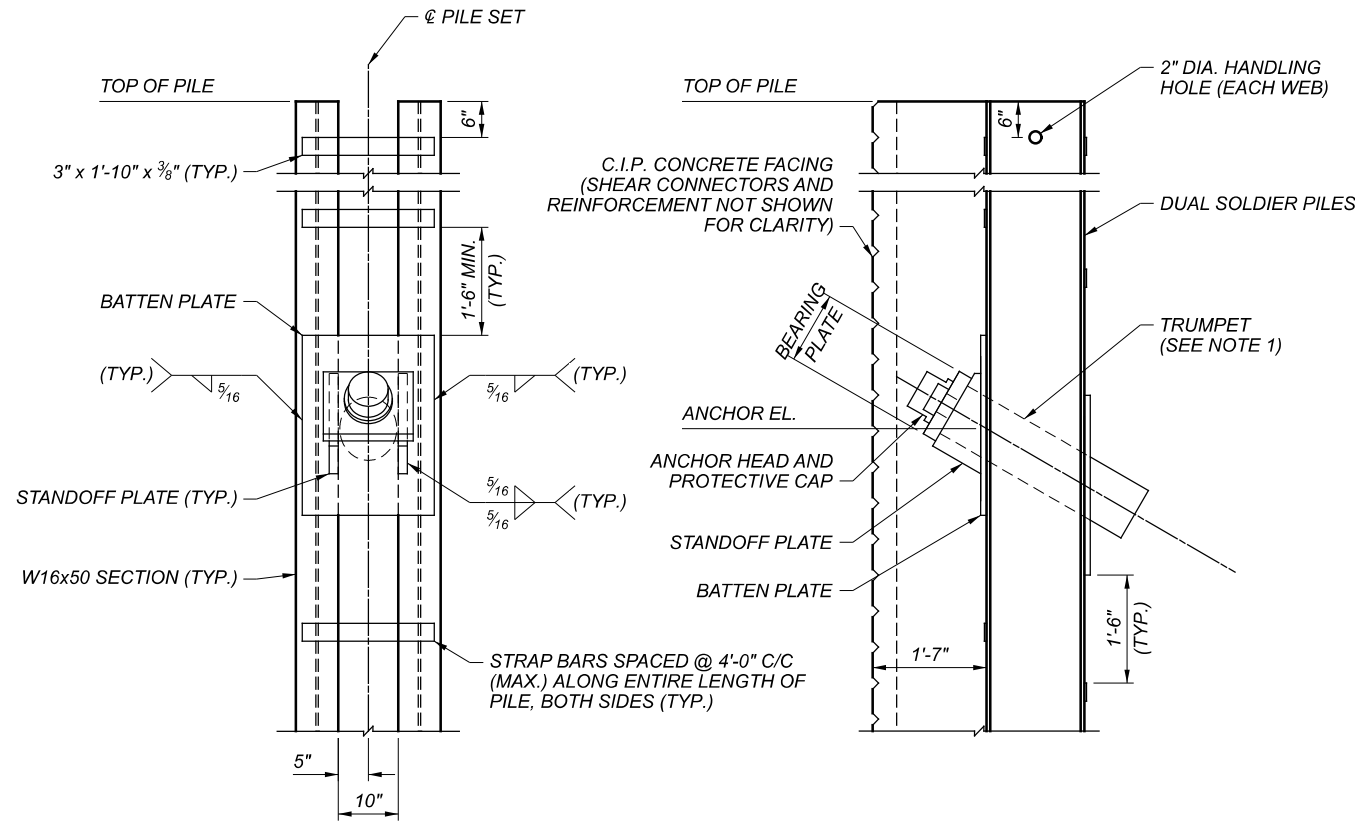
TANGENT DRILLED SHAFT CAP PLAN



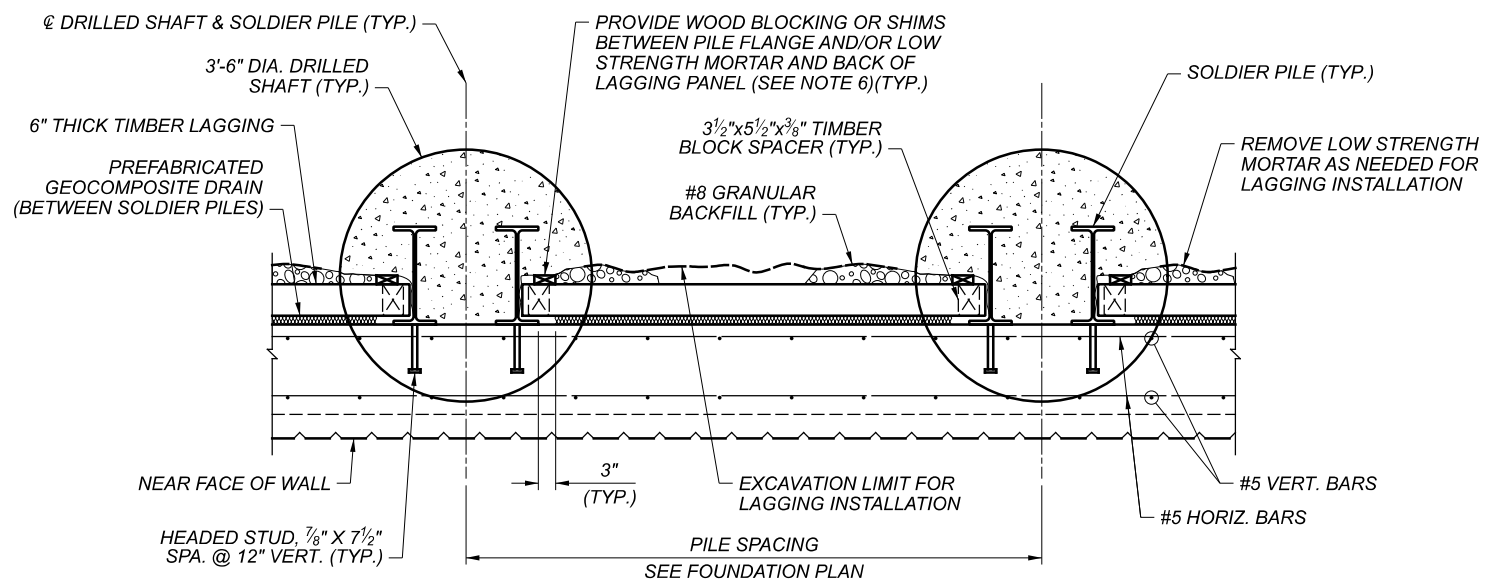
TANGENT DRILLED SHAFT CAP ELEVATION
 (DIMENSIONS GIVEN ALONG \perp CONST. WALL AG)

LEGEND:
 # - DENOTES SOLDIER PILE NUMBER

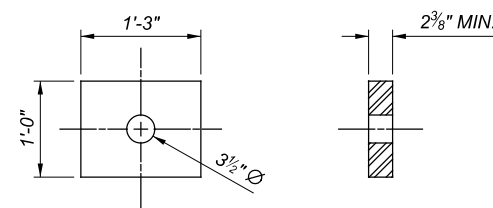
SFN	N/A
DESIGN AGENCY	Michael Baker INTERNATIONAL
DESIGNER	SSW
CHECKER	GZ
REVIEWER	LPC 08-04-22
PROJECT ID	82382
SUBSET	16
TOTAL	18
SHEET	1048
TOTAL	2338



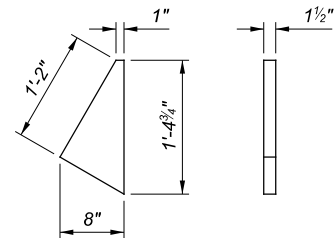
ANCHOR CONNECTION DETAIL



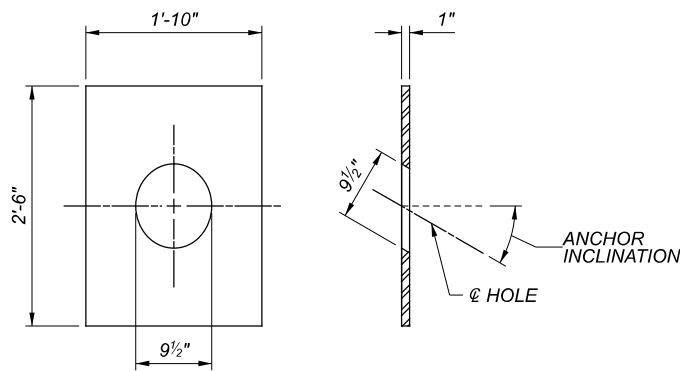
TYPICAL ANCHORED SOLDIER PILE WALL PLAN



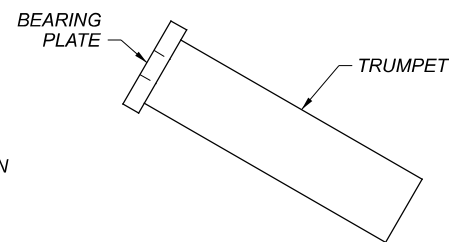
BEARING PLATE DETAIL



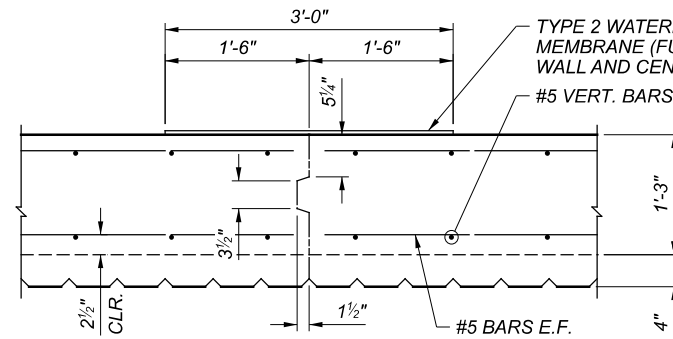
STANDOFF PLATE DETAIL



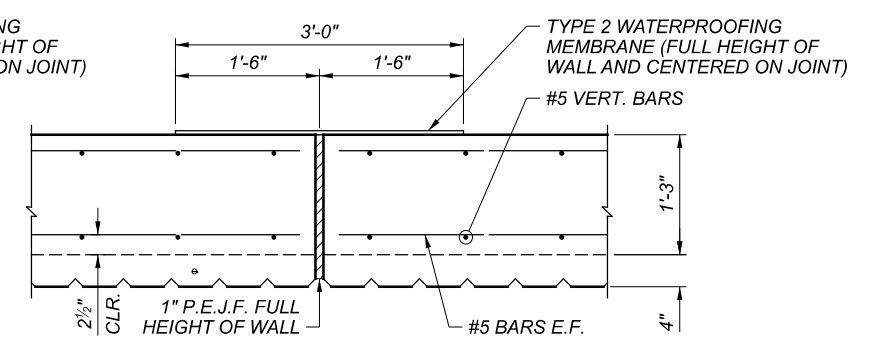
BATTEN PLATE DETAIL



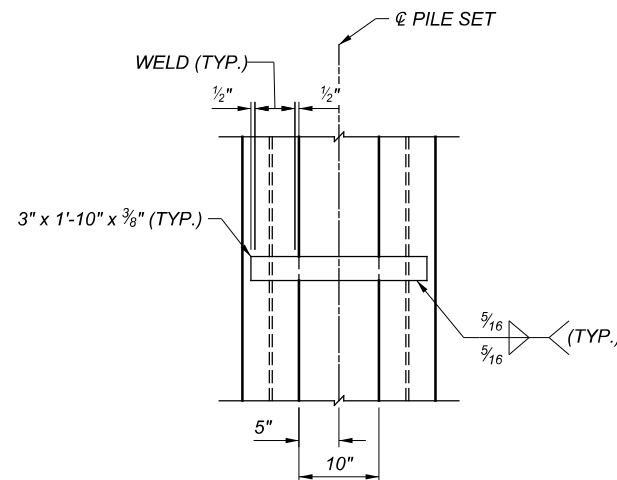
TRUMPET DETAIL
(SEE NOTE 1)



TYPICAL CONTRACTION JOINT DETAIL



TYPICAL EXPANSION JOINT DETAIL



PILE STRAP DETAIL

NOTES

1. SIZE, WALL THICKNESS AND STEEL GRADE OF TRUMPET TO BE DETERMINED BY CONTRACTOR BASED ON PROPOSED INSTALLATION METHOD.
2. SOLDIER PILES SHALL BE PRE-FABRICATED WITH THE ANCHOR DETAIL SHOWN BEFORE INSTALLATION INTO THE DRILLED CONCRETE SHAFT, BEARING PLATES, WEDGE PLATES, STIFFENERS, STEEL TRUMPET PIPE, GALVANIZING, AND ANY ADDITIONAL ANCHOR MODIFICATIONS TO THE SOLDIER PILES ARE INCLUDED WITH THE APPROPRIATE ITEM 866 - GROUND ANCHOR, AS PER PLAN FOR PAYMENT.

SFN	N/A
DESIGN AGENCY	Michael Baker INTERNATIONAL
DESIGNER/CHECKER	SSW / GZ
REVIEWER	LPC 08-04-22
PROJECT ID	82382
SUBSET	TOTAL
17	18
SHEET	TOTAL
1049	2338

