

Physical Condition Report for

CUY-006-1456

SFN 1800930

Detroit-Superior (Veterans Memorial) Bridge over the Cuyahoga River

2021 Routine Inspection



Prepared for:

Ohio Department of
Transportation

District 12

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Inspection Dates and Procedure

NBI Inspection Completion Date: 10/29/21

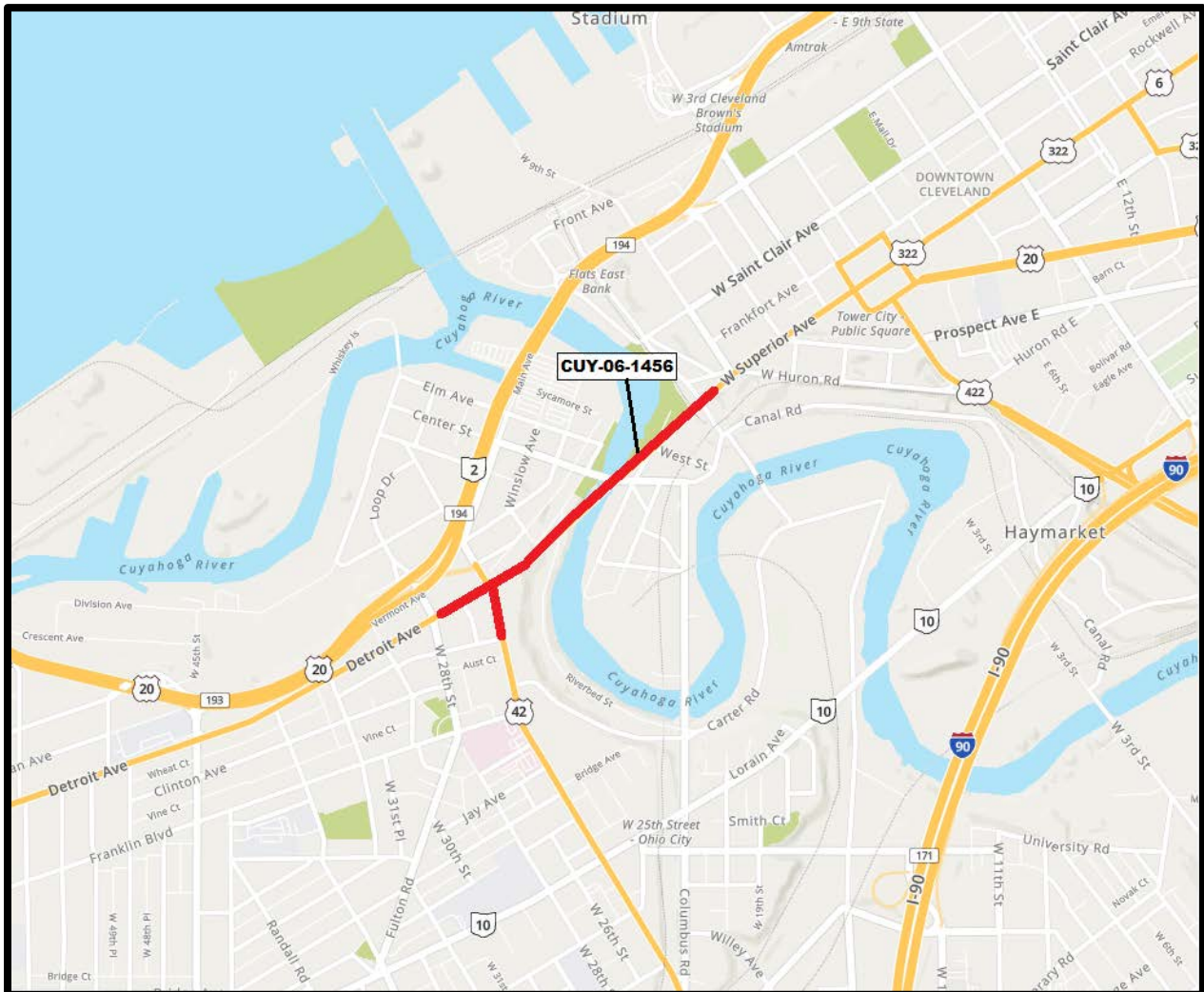
Palmer Engineering, with the assistance of Srinteg conducted a routine & fracture critical inspection on the structural elements of the bridge. The majority of the bridge elements were accessed using aerial work platforms. A 120-ft aerial work platform was used to access Spans 1-3 and 5-13 from the ground below. A 40-ft aerial work platform was used to access portions of Spans 1-13 from the maintenance (lower) deck. The Span 4 steel truss was accessed using industrial rope access techniques. The inspection crews documented previously reported and new areas of deterioration and structural distress.

The following personnel were involved in the bridge inspection:

- Justin Rufener, PE, Team Leader (Palmer)
- Matt Johnson, PE, Team Leader (Palmer)
- Adam Lenemier (Palmer)
- Pete Anamasi, PE, Team Leader (Patrick)
- Tim Tuskas, PE (Patrick)
- Don Cartwright, PE, Team Leader (Srinteg)
- Doug Dickson, PE, Team Leader (Srinteg)
- Luke Traverso, EI (Srinteg)

Previous construction and rehab that was ongoing during the 2020 inspection was completed prior to the 2021 inspection. Active construction work on Canal Basin Park limited access to Spans 7 & 8 from below the bridge.

Location Map



Structure: CUY-06-1456
Detroit-Superior/Veterans Memorial over the Cuyahoga River
Cleveland, OH

General Bridge Description

The Veterans Memorial/Detroit-Superior Bridge (CUY-6-1456, SFN 1800930) carries three lanes of vehicular traffic and one lane of bike traffic over the Cuyahoga River Valley, local streets, and RTA railroad tracks. The bridge is approximately 2,880 feet long, including 1,673 feet of subway tunnel that is linked by the lower deck. The bridge was constructed from 1912 to 1917.

The upper deck was opened to vehicular traffic in November 1917 and currently carries three lanes of traffic. The lower deck was designed for four streetcar lines with room for an additional two lines. The rail lines were active from January 1918 to 1953. On January 18, 1974, the bridge was added to the National Register of Historic Places. On Veterans Day, November 11, 1989, the bridge was renamed the Veterans Memorial Bridge.

The bridge consists of three (3) units of varying structure types within each unit:

Unit I – West Approach
Unit II – Main Unit Spans
Unit III – East Station

Plan views of the Veterans Memorial/Detroit-Superior Bridge with the units identified are shown in *Figures 1 - 3*.

Unit I – West Approach

The West Approach section consists of the West Station area spanning a total of 350 feet west of Tower A and two abandoned subway tunnels: the Detroit Avenue Tunnel (660 feet long) and the West 25th Street Tunnel (480 feet long). There are several utilities that pass through the west station and tunnels.

Unit II – Main Unit Spans

The Main Unit is composed of Spans 1A, 1B, and Spans 1 through 13. Spans 1A and 1B are transition structures from the underground West Station to the main spans. These two concrete cellular spans total 220 feet long and have an enclosed cellular construction below the lower deck referred to as the chambers. Spans 1 through 13 are the main spans of the bridge with a double deck design. Spans 1 through 3, 5 through 11, and 13 are concrete open spandrel arches. Span 12 is a concrete encased steel half through arch. Span 4 is a 591-foot long, three-hinged steel half through arch truss in a Pratt configuration. The upper deck is used for vehicular and pedestrian traffic and the lower deck is used for utilities and maintenance access.

Unit III – East Station

The East Station is a concrete cellular span that extends 165 feet past the East Abutment. A three-panel long, cellular construction is present under the East Station lower deck immediately behind the East Abutment.

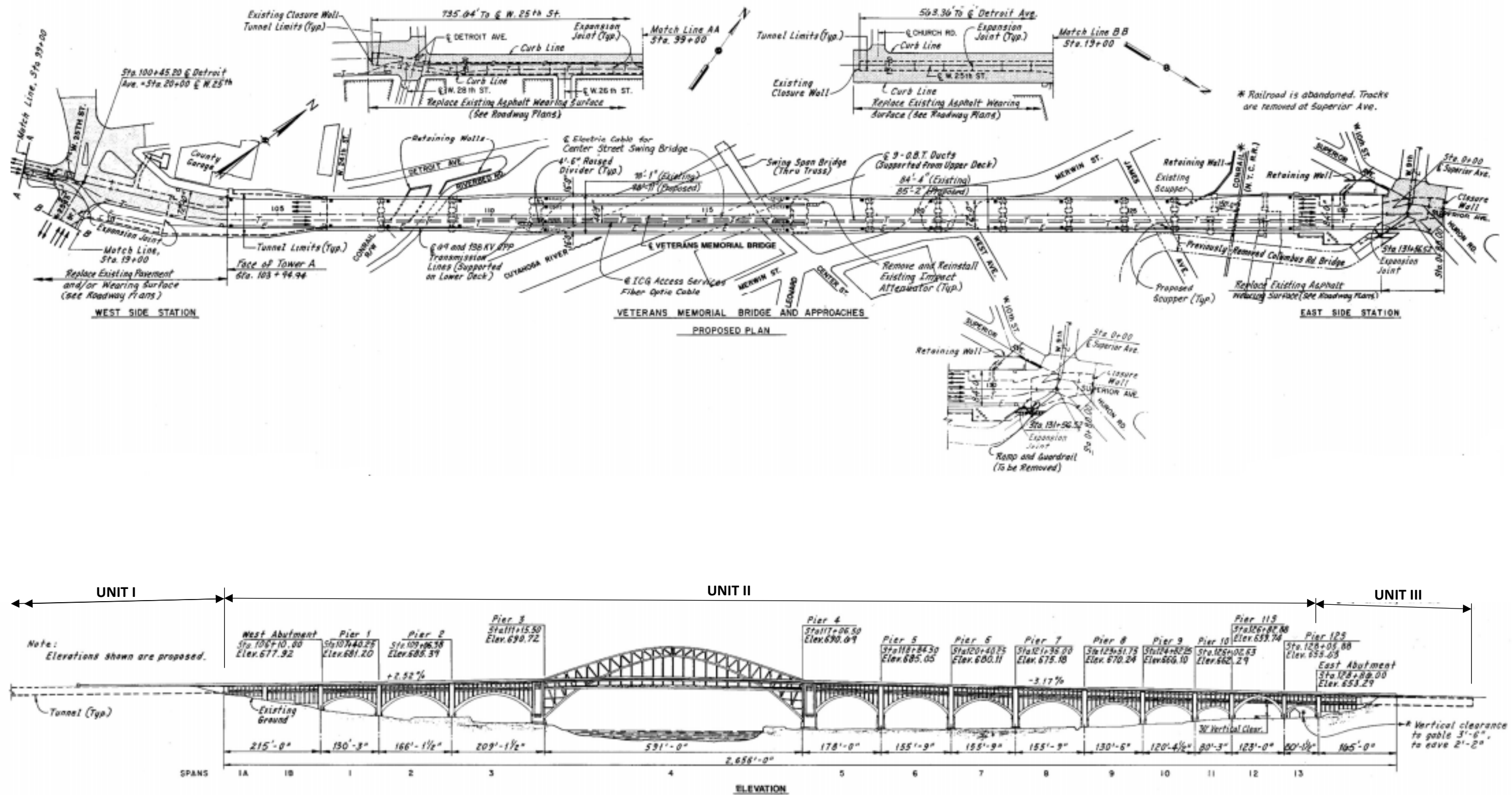


FIGURE 1: PLAN AND ELEVATION VIEW

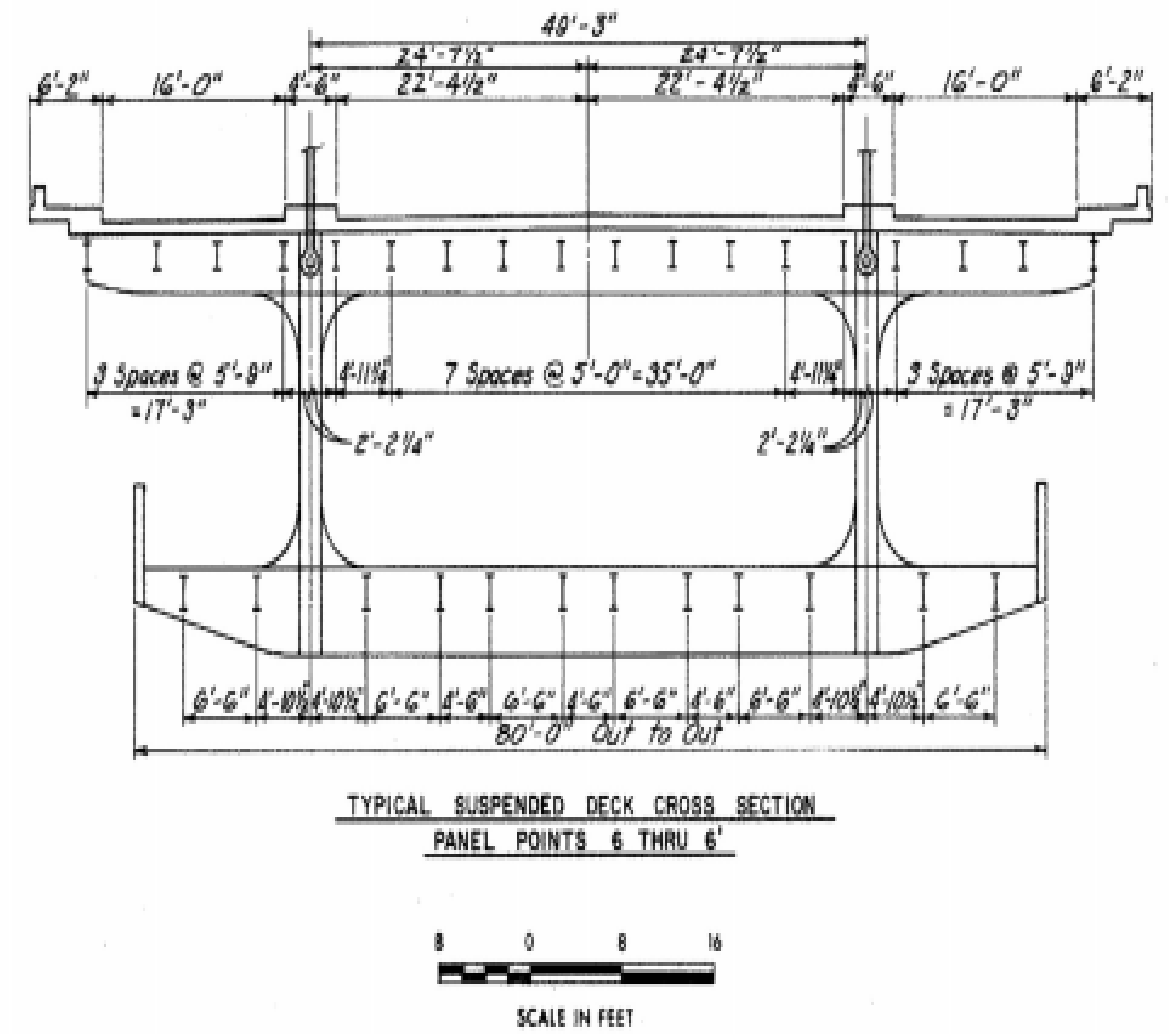
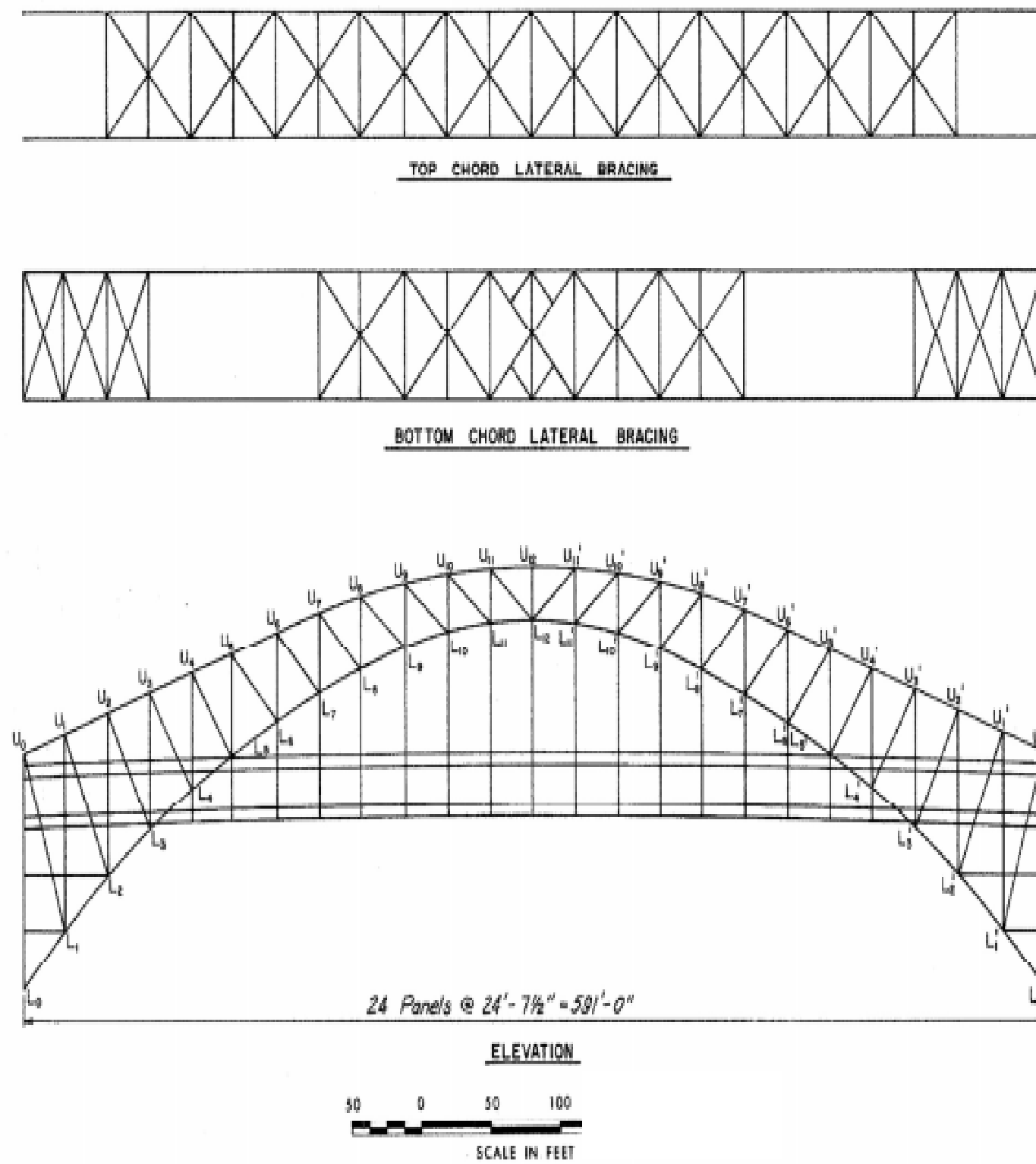


FIGURE 2: SPAN 4 NOMENCLATURE

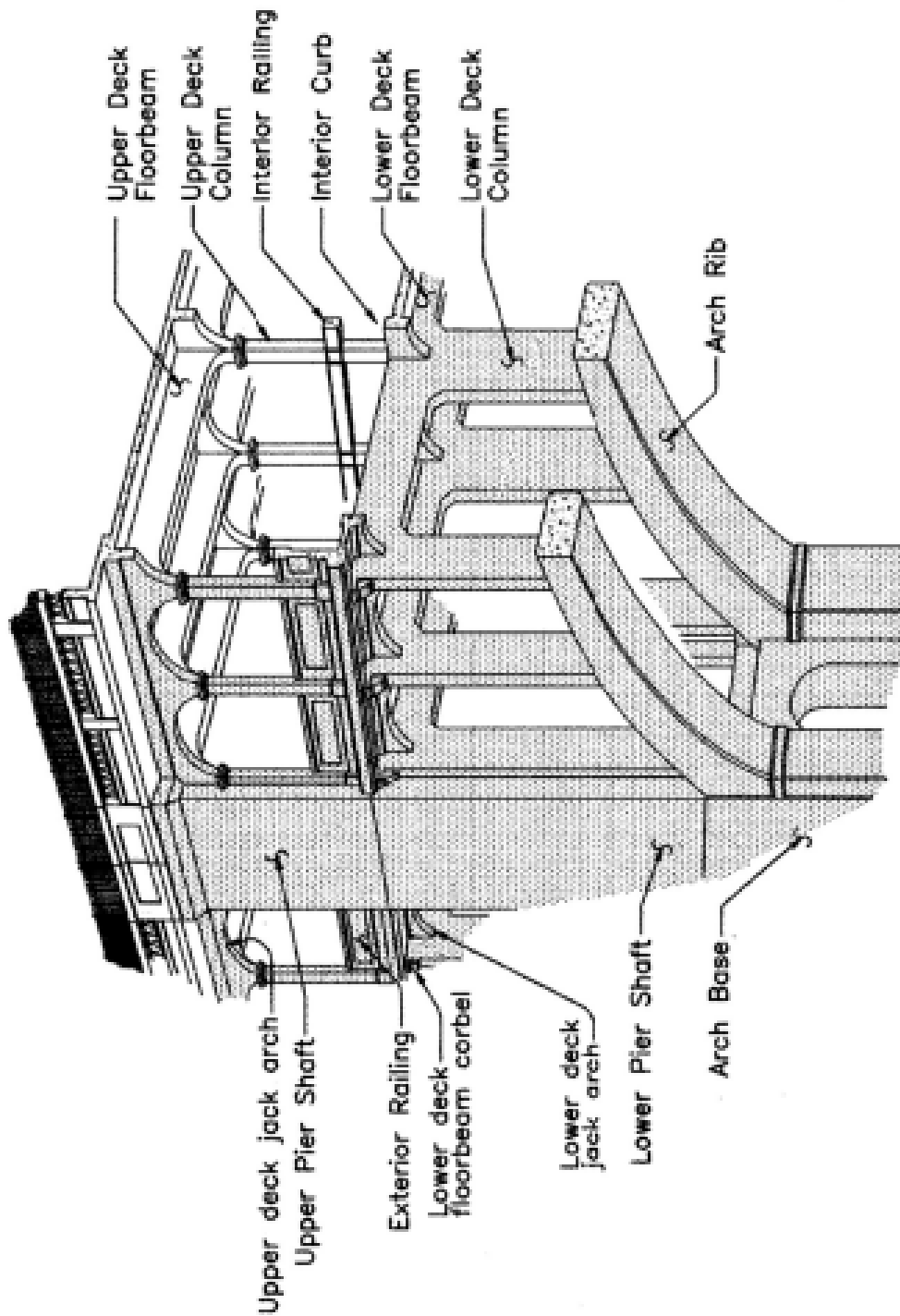
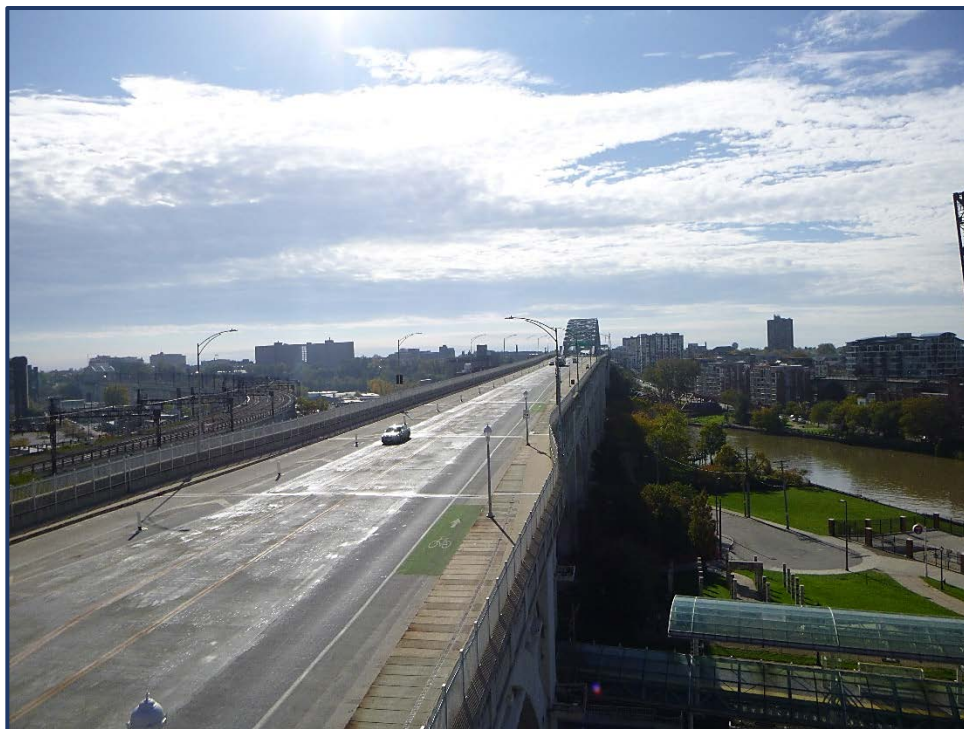


Figure 3: Upper and Lower Deck Nomenclature, Concrete Arch Spans

Structure Typical Photos



West End of Bridge, Looking East



East End of Bridge, Looking West



Main Truss Span (Span 4), Looking West



South Elevation of Bridge, Looking Northwest



North Elevation of Bridge, Looking Southeast



North Elevation of Span 4, Looking South

Construction and Maintenance History

The following is a summary of significant events in the history of the Detroit-Superior bridge:

- 1914-1917: Construction of the Detroit-Superior High Line Bridge
- November 1917: Bridge opened up to vehicular and pedestrian traffic.
- January 1918: Bridge opened up to streetcar traffic.
- 1953: Streetcar lines abandoned.
- 1967-70 Major Rehabilitation
 - Removal of the original upper deck consisting of four vehicular lanes and two 15-foot wide sidewalks.
 - Strengthening or replacement of all upper deck concrete floorbeams.
 - Span 4: Erection of new steel floorbeam cantilevers.
 - Construction of the new upper deck with six vehicular lanes and two 5-foot wide sidewalks.
- January 18, 1974: Bridge was added to the National Register of Historic Places
- November 11, 1989: Bridge was renamed the Veterans Memorial Bridge.
- 1995-97 Major Rehabilitation
 - Replacement of the upper and lower deck floors.
 - Replacement of select upper and lower concrete floorbeams, columns, jack arches and pier shafts.
 - Application of epoxy-urethane or non-epoxy sealer to most exposed concrete surfaces.
 - Span 4: Replacement of all steel hangers, Panel Points 6 through 6'.
 - Span 4: Replacement of upper deck and lower deck Floorbeams 5 through 5' and the corresponding stringers.
 - Painting of all steel superstructure components.
 - Installation of new drainage system.
 - Installation of architectural lighting.
- 2003 North Sidewalk Linear Park Conversion.
 - Conversion of vehicular traffic to two westbound and one eastbound lane between the steel trusses and one eastbound lane on the Span 4 south cantilever.
 - Widening of the north sidewalk with longitudinal trench drainage.
 - Installation of public art and benches along the modified north sidewalk.
- 2014-2018
 - Span 1A through Span 13: Patching deficient upper deck wearing surface areas.
 - Patch deficient concrete super and substructure components in West Station, Detroit Avenue Tunnel, West 25th Street Tunnel and Spans 1-3 and 5-13. (Note: In Spans 1-3 and 5-13, the patching below the lower deck was later restricted to areas adjacent to and over public areas.)
 - Span 4: Zone painting of primary and secondary truss members between upper and lower decks.

- Installed hanger caps at hanger opening in upper deck, Panel Points 6 through 6'.
- Repaired spalled wearing surface in Span 9.
- Drain pipe cleanout for pedestrian tunnels in the west and east station. (This task was performed but not successful.)
- South vehicular eastbound lane converted into bike lane.
- 2019 – 2021
 - Replacement of wearing surface with new micro silica modified concrete overlay.
 - Concrete patching and crack sealing in concrete arch spans and approach tunnels.
 - Fiber wrapping in select locations in concrete arch spans over public areas.

Condition and Element Rating Guidelines

The table below contains the bridge inspection rating matrix established by the Federal Highway Administration (FHWA), and employed by ODOT, using a 0-Failed through 9-Excellent scale. The General Appraisal, Deck, Wearing Surface, Expansion Joint, Superstructure, Protective Coating System, Substructure, Channel and Scour rating will follow these 0 through 9 rating guidelines.

Summary Items (NBIS)	Condition	Defect
9	Excellent	Excellent condition.
8	Very Good	No problems noted.
7	Good	Some minor problems.
6	Satisfactory	Structural elements show minor deterioration.
5	Fair	All primary structural elements are sound but may have minor section loss, crackling, spalling or scour.
4	Poor	Advanced section loss, deterioration, spalling or scour.
3	Serious	Loss of section, deterioration, spalling or scour has seriously affected primary structural components. Local failures are possible. Fatigue cracks in steel or shear cracks in concrete may be present.
2	Critical	Advanced deterioration of primary structural elements, Fatigue cracks in steel or shear cracks in concrete may be present or scour may have removed substructure report. Unless closely monitored it may be necessary to close the bridge until corrective action is taken.
1	"Imminent Failure"	Major deterioration or section loss present in critical structural components or obvious vertical or horizontal movement affecting structure stability. Bridge is closed to traffic but corrective action may be put it back in light service.
0	Failed	Out of service – beyond corrective action.

Manual of Bridge Inspection, Ohio Department of Transportation (ODOT), 2014 (with 2017 & 2021 Addendums)

Bridge Inspector's Reference Manual, Federal Highway Administration (FHWA), 2015

Manual for Bridge Element Inspection, 2nd Edition, AASHTO, 2019

National Bridge Inspection Standards, U.S. Department of Transportation, 2004

Inspection of Fracture Critical Bridge Members, U.S. Department of Transportation, 1986

Inspection Findings

Inspection findings are presented below. Findings shown in **RED** text indicate new findings during the 2021 inspection.

NBI Item N58 – Deck (6, Satisfactory Condition)

The deck is in overall **Satisfactory** condition. There are two NBI sub-items under the deck condition:

NBI Item N58.01 – Wearing Surface (8, Very Good Condition)

NBI Item N58.02 – Expansion Joints (6, Satisfactory Condition)

The deck findings and summary of deck conditions for individual deck elements are as follows:

Element 12 – Reinforced Concrete Deck

The reinforced concrete deck is in **Satisfactory** condition. The deck is divided into several sections as detailed below:

Detroit Avenue Tunnel: During the 1995-1997 rehabilitation, a new reinforced concrete slab was placed on top of the original slab. The new slab was designed to support live and dead loads, with the original slab offering no structural support. The top and bottom surfaces for the new slab are not visible and assumed to be in good condition, despite the poor and critical conditions of the original tunnel slab beneath (*Photo 12-1*).

West 25th Street Tunnel: The West 25th Street tunnel ceiling is in satisfactory condition, with areas of saturation, isolated delaminated areas and some shallow spalling with exposed reinforcing

West Station: The West Station ceiling is in fair condition and has areas of spalling, cracking and efflorescence, active water infiltration, and exposed reinforcing steel (*Photo 12-2*).

Spans 1A, 1B, and 1 through 13: The upper deck floor in the main spans is in satisfactory condition. There are isolated cracks with some efflorescence, sound and unsound patches and spalls, some with exposed reinforcing (*Photo 12-3*). There are numerous areas of moisture staining, some of which have mottling (*Photo 12-4*).

East Station: The East Station ceiling is overall in good condition with scattered cracking with efflorescence.

Lower Deck: The lower deck floor is not open to vehicular or pedestrian traffic and is therefore not included as part of the element quantities. The lower deck floor is in good condition and consists of reinforced concrete with metal stay-in-place forms in Spans 1

through 3, and Spans 5 through 13. In isolated locations, the stay-in-place forms have active corrosion. In Span 4, the lower deck is an open steel grid type in middle section, and fiberglass grid in the exterior sections.

Refer to *Open Spandrel Arch Drawings in Appendix C* for specific deficiencies and locations.

	Total Quantity	CS 1	CS 2	CS 3	CS 4
Detroit Ave. Tunnel	17,950 SF	17,950 SF			
West 25 th St Tunnel	13,750 SF	12,450 SF	1,100 SF	200 SF	
West Station	37,800 SF	24,800 SF	11,000 SF	2000 SF	
Spans 1A, 1B, 1-13	232,250 SF	220,538 SF	11,612 SF	100 SF	
East Station	31,150 SF	30,838 SF	312 SF		
Total Structure	332,900 SF	306,576 SF	24,024 SF	2300 SF	

Element 300 – Strip Seal Expansion Joint

The expansion joints are overall in **Satisfactory** condition. Joints typically have sections with loose debris (*Photos 300-1*). On the sidewalks, there are locations of edge spalls along the joint armor (*Photo 300-2*). There are some scattered areas of active leakage. At the joint at Pier 10, there is a 4’ long section of tearing seal in the eastbound lanes (*Photo 300-3*), and minor damage to joint armor. **At Joint 1A, there is a 2’ long section of broken intermittent fillet welds in the joint armor in the eastbound lanes (*Photo 300-4*).** Refer to Table 1 for joint measurements and Table 2 in *Appendix B* for specific deficiencies.

Total Quantity	CS 1	CS 2	CS 3	CS 4
2,579 LF	1479 LF	1000 LF	100 LF	

Element 330 – Metal Bridge Railing

The median railings are in **Good** condition. The median railings are located along the edges of the roadway in Span 4 to protect the truss and hangers from vehicle impact.

Total Quantity	CS 1	CS 2	CS 3	CS 4
1366 LF	1366 LF			

Element 331 – Reinforced Concrete Bridge Railing

The concrete railings are in **Good** condition. The railings on the north and south side of the bridge consist of a reinforced concrete railing with an aluminum fence on top. All concrete railing is in good condition with minor cracking, staining, and isolated spalling (*Photo 331-1*). The metal fence has isolated areas of minor damage (*Photo 331-2*). Refer to Table 2 in *Appendix B* for specific deficiencies.

Total Quantity	CS 1	CS 2	CS 3	CS 4
5,312 LF	5,158 LF	150 LF	4 LF	

Element 510 – Wearing Surface

The wearing surface is in **Very Good** condition. In Spans 1A – 13 and the East Station, the wearing surface is a micro silica modified concrete, which was placed in 2019. There are isolated locations of map cracking in the concrete wearing surface (*Photos 510-1 to 510-2*). Above the Detroit Avenue Tunnel, West 25th Street Tunnel and West Station, the wearing surface is asphalt. This asphalt has areas of transverse and map cracking (*Photos 510-3*).

	Total Quantity	CS 1	CS 2	CS 3	CS 4
Concrete	177,255 SF	175,255 SF	2000 SF		
Asphalt	69,500 SF	66,025 SF	3475 SF		
Total Structure	246,755 SF	241,280 SF	5475 SF		

Element 815 – Drainage

The deck drainage is in **Fair** condition. The West Abutment south downspout **and the Pier 1 south downspout** are completely clogged at the base of the catch basin (*Photo 815-1*). At Pier 3, the downspout is disconnected at the base of the pier, allowing drainage on the pier face (*Photo 815-2*). The Pier 9, south catch basin concrete frame has shifted and the north catch basin cover has shifted and rotated. The north sidewalk longitudinal trench drains are filled with debris and not functioning (*Photo 815-3*). Some of the scupper inlets are fully or partially clogged (*Photo 815-4*). At Pier 5 on the North Side, **and in Span 13 on the South Side** (*Photo 815-5*), there is active leakage coming through the utility entrances in the deck, allowing roadway drainage onto the maintenance deck. Refer to Table 2 in *Appendix B* for specific deficiencies.

Total Quantity	CS 1	CS 2	CS 3	CS 4
28 EA	20 EA	4 EA	2 EA	2 EA

Curb/Sidewalk

The concrete curb and sidewalk are in **Satisfactory** condition. The curbs and sidewalks have areas of cracking, delamination, and spalling (*Photos CS-1 to CS-2*). The steel curb plates have widespread surface corrosion. Refer to Table 2 in *Appendix B* for specific deficiencies.

NBI Item N59 – Superstructure (5, Fair Condition)

The superstructure is in **Fair** condition. There is an NBI sub-item under the substructure condition:

NBI Item N59.01 – Protective Coating System (6, Satisfactory Condition)

The protective coating system is in **Satisfactory** condition.

The superstructure findings and summaries of conditions for individual items are as follows:

Concrete Superstructure

Element 110 - Reinforced Concrete Beam

The beams are in overall **Fair** condition. This element consists of the longitudinal beams in the Detroit Avenue Tunnel, West 25th Street Tunnel, and West Station. The concrete beams have delaminations, efflorescence, and some areas of spalling with exposed reinforcing (*Photo 110-1*). Refer to Table 7 in *Appendix B* for specific deficiencies.

Total Quantity	CS 1	CS 2	CS 3	CS 4
7,394 LF	6,244 LF	1,000 LF	150 LF	

Element 144 – Reinforced Concrete Arch

The concrete arches are in **Fair** condition. This element encompasses the concrete arches and arch columns (*Photo 144-1*). The concrete arches in Spans 5 through 10, Span 13 and portions of Span 3 were patched, crack injected and then wrapped on the underside with FRP to prevent future spalling (*Photo 144-2*). Select columns were also patched. The concrete arches and columns that were not repaired typically have areas of cracking, delamination, poor patching and spalling with and without exposed reinforcing (*Photos 144-2 to 144-6*). The concrete jack arches connecting the columns below the upper and maintenance decks have spalls with exposed reinforcing steel, cracks, and delaminated areas (*Photo 144-7*).

Refer to *Open Spandrel Arch Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
8,040 LF	6,7400 LF	1200 LF	100 LF	

Element 155 – Reinforced Concrete Floor Beam

The concrete floorbeams in Spans 1A, 1B, 1 through 3, and Spans 5 through 13 are in **Satisfactory** condition. The floorbeams have isolated spalls with and without exposed reinforcing, cracking, delaminations, and areas of poor patching (*Photos 155-1 to 155-3*). The lower deck floorbeams tend to be in worse condition than the upper deck floorbeams, however they do not support traffic loading. The structural corbels are included in the rating of this element and exhibit similar defects as the rest of the floorbeams. The lower deck floorbeams in the East Station have the bottom mat of reinforcing steel exposed (*Photo 155-4*). This deterioration has changed little since the 1980s, but they carry no substantial live load. As part of the most recent rehabilitation, the undersides of lower deck floorbeams in Spans 5 through 10, 13 and portions of Span 3 were fiber wrapped after patching, to prevent future spalling (*Photo 155-5*).

Refer to the *Floor System Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
33,543 LF	22,543 LF	10,000 LF	1000 LF	

Steel Superstructure (Span 4)

The load bearing components (web plates and flange angles) of the primary truss members and gusset plates are composed of nickel steel, an early high strength steel also known for its corrosion resistant properties. The original hangers, also composed of nickel steel, were replaced with 50 ksi low carbon steel. All lacing member components of the primary truss members, upper and lower deck floorbeams, lateral and longitudinal bracing and sway bracing are composed of 30 ksi carbon steel.

Element 113 – Steel Stringer

The stringers are in **Satisfactory** condition. There are 18 lines of stringers in the upper deck and 12 in the lower deck. The upper deck stringers have shear studs welded to the top flange providing composite action with the deck. The upper and lower deck stringers in Panels 4, 5, 5', and 4' were replaced in 1995. The upper deck stringers are in good condition with some areas of active corrosion and pitting at the ends (*Photo 113-1*). The

original curb stringers of Lines E and N have areas of painted over pitting and some corrosion holes, with some active corrosion throughout their length (*Photos 113-2 to 113-3*).

The lower deck is not open to vehicular or pedestrian traffic; the stringers are therefore not included as part of the element quantities. The stringers supporting the steel grid deck are in good condition. Stringers D, E, I and J, which support only their own dead load, have areas of painted over advanced section loss and perforations at the floorbeams and saddle bearings. Stringer K, which supports the south fiberglass pedestrian deck, has similar locations of advanced section loss (*Photo 113-4*). The rest of the stringers supporting the outer pedestrian fiberglass grid deck are in good condition.

Refer to Tables 3 & 4 in *Appendix B* and *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
10,638 LF	10,338 LF	200 LF	100 LF	

Element 120 – Steel Truss

The steel truss is overall in **Satisfactory** condition. There are areas of pack rust, pitting, surface corrosion, and rivet head loss, which are worse at and below the upper deck (*Photos 120-1 to 120-3*). Advanced section loss and perforations, many of which have been cleaned and painted over, are present in the batten plates and lacing bars (*Photos 120-4 to 120-5*). At the eyebar connections, there are areas of painted over and reactivated pitting in the web plates (*Photo 120-6*). Refer to *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
1182 LF	490 LF	500 LF	192 LF	

Element 152 – Steel Floor Beam

The steel floorbeams are in **Satisfactory** condition. The floorbeams typically have painted over perforations near the deck openings at the truss lines, with repair plates welded in place at some of these locations (*Photos 152-1 to 152-4*). Active surface corrosion is present due to ongoing water infiltration at the deck openings.

On lower deck Floorbeams 10, 12 and 11' through 6', there are cracks along the weld of the stiffening plates to the top flange at the north truss line (*Photo 152-5*). **Some of the**

cracks have increased in length from the 2020 Inspection. The cracks at Floorbeam 12 were not noted in previous inspections.

Refer to Tables 3 & 4 in *Appendix B* and *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
3,925 LF	3,575 LF	200 LF	150 LF	

Element 161 – Steel Pin & Hanger Assembly

The pins, hangers and hinges are in **Good** condition with no significant deficiencies noted. Minor painted over pitting was noted on some eye-bars below the upper deck (*Photo 161-1*). There is active corrosion on some of the hangers above the previous zonal painting. Refer to Tables 3 & 4 in *Appendix B* and *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
30 EA	27 EA	3 EA		

Element 162 – Steel Gusset Plates

The truss gusset plates are in **Fair** condition. The gusset plates typically have areas of active surface corrosion. The lower chord gusset plates below the upper deck typically have painted over pitting and reactivating corrosion along the top of the lower chord (*Photos 162-1 to 162-2*). In other some locations, the gusset plates have areas of pitting and rivet head loss. Refer to *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
100 EA	20 EA	47 EA	33 EA	

Lateral Bracing & Sway Bracing

The lateral bracing and sway bracing is in **Satisfactory** condition with isolated areas of active surface corrosion, pack rust, and advanced section loss including perforations

(Photo BR-1). Refer to *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Element 313 – Fixed Bearing

The bearings are in **Fair** condition with some pack rust around the pins, and surface corrosion noted on the interior faces of all four bearing castings (Photo 313-1). The non-structural bearing pin cover plates have cracks up to 7 inches long at L0 and L0' on both trusses (Photo 313-2). The north pin cover at L0 on the north truss has broken off (Photo 313-3). There is advanced section loss of some of the anchor bolt and nuts (Photo 313-4).

Between the deck underside and the top of the transverse floorbeams over Piers 11 & 12, there are 3" H concrete pedestals with galvanized steel plates sitting on top and between each pedestal. In several locations, these plates are missing and no longer support the deck underside, or are have varying displacements from the bearing pedestal (Photo 313-5).

Refer to *Truss Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
4 EA		4 EA		

Item 515 – Steel Protective Coating

The protective coating system (PCS) is in **Satisfactory** condition. Areas of corrosion, peeling and failed paint are present on the main truss members below the lower deck (Photo 515-1). The structural steel between the upper and lower decks was repainted in 2014-2015 and is in good condition. The protective coating system above the upper deck has surface corrosion with minor rust staining.

Total Quantity	CS 1	CS 2	CS 3	CS 4
214,850 SF	190,340 SF	13,780 SF	10,130 SF	600 SF

Fatigue Prone Details

The fatigue prone details are in **Fair** condition. Stiffening retrofit plates welded to the top flange of the lower deck floorbeams at the truss lines are classified as Category E fatigue details. Cracks in the fillet welds are present at several locations. Refer to

Element 152 – Steel Floor Beam above for additional details on crack locations and growth.

NBI Item N60 – Substructure (6, Satisfactory Condition)

The substructure is in overall **Satisfactory** condition.

The substructure findings and summary of conditions for individual items are as follows:

Element 205 – Reinforced Concrete Column

The pier columns are in **Satisfactory** condition. This item includes the main span pier columns, columns in Spans 1A and 1B, and the columns in the subway tunnels and stations. The main span pier columns have areas of map cracking, failing patching, and delaminations (*Photos 205-1 to 205-2*). There are also a few areas of minor spalling with exposed reinforcing. The decorative arches above these columns have areas of map cracking, failing patching, delamination and spalling with exposed reinforcing. The remaining columns have areas of failing patching, delamination, and spalling with and without exposed reinforcing (*Photos 205-3 to 205-4*).

The decorative towers on the north and south faces of the piers typically have widespread cracking, delamination and spalling with and without exposed reinforcing (*Photo 205-5*). The south towers at Piers 5 through 7 are leaning away from bridge, with gaps up to 2¼-in between the top of the tower and the outside face of the upper jack arches (*Photo 205-6*). Detailed measurements of the gaps between these towers and the adjacent bridge features are given *Table 5* in *Appendix B*. No change was noted between the current measurements and those taken during the 2020 inspection.

Refer to *Table 7* in *Appendix B* and *Substructure Drawings* in *Appendix C* for specific deficiencies and locations.

	Total Quantity	CS 1	CS 2	CS 3	CS 4
Main Span Piers	40 EA	25 EA	15 EA		
Spans 1A & 1B	108 EA	35 EA	25 EA	48 EA	
Tunnels & Stations	365 EA	280 EA	60 EA	25 EA	
Total Structure	513 EA	340 EA	100 EA	73 EA	

Element 210 – Reinforced Concrete Pier Wall

The pier walls at Piers 1, 3 and 4 are in **Satisfactory** condition. The west face of Pier 1 is primarily covered by fill. The exposed portions of the pier walls have areas of map cracking and delamination (*Photo 210-1*). Piers 3 and 4 are located adjacent to the

Cuyahoga River. The portion of Pier 3 that is exposed to the channel has widespread areas of deep abrasion and numerous spalls (*Photo 210-2*). Piers 3 and 4 are cellular type structures, which are open on their Span 3 and 5 faces, respectively. The interiors faces of the walls have areas of delamination and spalling with exposed reinforcing (*Photo 210-3*). Refer to *Substructure Drawings* in *Appendix C* for specific deficiencies and locations.

Total Quantity	CS 1	CS 2	CS 3	CS 4
200 LF	100 LF	50 LF	50 LF	

Element 215 – Reinforced Concrete Abutment

The abutment walls are in **Satisfactory** condition. The abutment walls consist of the West and East Abutments and the walls of the Detroit Avenue and West 25th Street Tunnels. The abutments have areas of cracking with minor moisture staining, delamination and some spalling (*Photos 215-1*). Some staining appears to be superficial due to leaking deck joints above. In the tunnels, the lower 1-ft to 2-ft of the walls have widespread shallow spalling with exposed reinforcing (*Photo 215-2*). The portions of the walls in the tunnels above these areas were repaired as part of the most recent rehabilitation.

Total Quantity	CS 1	CS 2	CS 3	CS 4
3,459 LF	3,159 LF	200 LF	100 LF	

Element 830 – Abutment Backwall

The backwalls are in **Good** condition. The backwalls consist of the closure panels at the ends of the West 25th Street Tunnel, Detroit Avenue Tunnel, and East Station.

Total Quantity	CS 1	CS 2	CS 3	CS 4
263 LF	263 LF			

Wingwalls

The wingwalls are in **Poor** condition. The wingwalls along Spans 1A and 1B and the East Station have cracking and spalling with exposed reinforcement throughout (*Photo WW-1*). Refer to *Substructure Drawings* in *Appendix C* for specific deficiencies and locations.

Tower B South

A section of the rear abutment, south wingwall at Tower B has through cracks in the wall and associated footing, and the tower and cracked section of the wall are leaning to the

south (*Photo WW-2*). It has continued to show incremental movement over the past 10+ years. On the interior, the top of the tower is spalled and cracked due to contact with the soffit of the upper level sidewalk. Crack gages have been placed at several locations to monitor the movement of the section. Crack gauges located at the base of Tower B are cracked and slightly displaced. New gauges should be installed to ensure an accurate record of the tower rotation is maintained. Refer to Table 6 in *Appendix B* for detailed defect drawings and crack gage measurements.

West and East Abutment Chambers

The chambers below Spans 1A and 1B on the west approach and below the East Station were inspected, however, they are not included in any of the quantities within this report. Horizontal, vertical, diagonal and map cracking with efflorescence and moisture staining are present throughout all cells (*Photo AC-1*). Areas of delaminations and spalling with exposed reinforcing are present in scattered locations. The floors are typically covered in dirt and construction debris. In the west chamber, there is heavy cracking around south Tower B (see discussion above for more details), and there are crack gauges present at several other locations.

Water Infiltration

Standing water was noted in the pedestrian tunnel under the West Station. In the east abutment chamber, most of the lower cells are filled with standing water. The pedestrian tunnel under the East Station is also filled with standing water.

Slope Protection

The slope protection is in **Satisfactory** condition, with some areas of erosion and sliding material noted.

NBI Item N61 –Channel (6, Satisfactory Condition)

The Channel is in **Good** condition. There is an NBI sub-item under the Channel condition:

NBI Item N61.01.01 – Scour (7, Good Condition)

The bridge scour is in **Good** condition.

The channel findings and summary of conditions for individual items are as follows:

Alignment

The alignment is in **Good** condition. The channel is skewed with respect to the piers, but this is an as-built condition (*Photos CH-1*).

Protection

The channel protection is in **Satisfactory** condition with only minor deficiencies. The west bank is vegetated with some dumped rock channel protection. The east bank is protected by a sheet pile wall.

Hydraulic Openings

The hydraulic opening is in **Good** condition with no major constrictions associated with the bridge.

Navigation Lights

The navigation lights are in **Poor** condition. None of the six lights were functioning at the time of inspection. No damage was noted to the light fixtures.

Scour

The scour is in **Good** condition. An underwater bridge inspection was performed on July 8, 2020. No areas of exposed foundation or significant scour holes were found in the inspection.

Approaches

The approaches are in **Satisfactory** condition.

The approach findings and summary of conditions for individual items are as follows:

Approach Wearing Surface

The approach wearing surfaces are in **Satisfactory** condition. There are some areas of transverse and map cracking.

Embankment

The approach embankments are in **Fair** condition. The embankment under Spans 1 through 3 has several slope depressions (*Photo APR-1*). This embankment was primarily loose soil placed over demolition debris. Beneath this fill are two concrete struts between Pier 2 and 3 used to maintain stability during construction. The south strut is preventing portions of the fill from sliding into the Cuyahoga River. This embankment is being monitored with slope inclinometers maintained by ODOT District 12.

The embankment along the south side of Spans 1A and 1B has significant erosion for the full length. At the west end of the erosion, there is a 15' diameter x 4' deep erosion ditch around a manhole (*Photo APR-2*). An erosion ditch extends from the manhole towards the east typically 3' W x 2' D. This erosion is relatively unchanged from the 2020 inspection. Tower B South, which is in this area, is leaning due to slope instability, as previously discussed.

Refer to *Substructure Drawings* in *Appendix C* for specific deficiencies and locations.

Guardrail

The approach guardrails are in **Good** condition with some minor impact scrapes in the concrete rail.

Security Items

There are locations where the structure and structure right-of-way can be accessed by non-bridge personnel. The fence which encloses the area under Span 1 and along the south sides of Spans 1A and 1B is accessible due to an unlocked gate on the southeast end of Pier 1, **and two locations where holes have been cut into the fence on the south side of Spans 1A & 1B (Photos APR-3 to APR-5)**. Due to these openings, there are multiple homeless encampments within the fenced in area. **Preventative access steel mesh installed outside Span 1A near Tower A to prevent access has failed (Photo APR-6)**. Security fencing installed around Piers 2 and 3 can easily be surpassed, and there is evidence of a homeless encampment inside of Pier 3.

A chain link enclosure for the Center Street Bridge operator's vehicle on the west side of Pier 4 allows vandals to climb the fencing cover to access the Span 4 truss. From here the vandals have vandalized Pier 4 and have access to the truss lower chord and potentially the lower deck.

Signs & Utilities

Signs

The signs on the structure are in **Good** condition.

Utilities

The utilities are in **Satisfactory** condition. The lower deck telephone junction chambers and supports are corroded due to saltwater infiltration through the manhole above (*Photo UT-1*). At Pier 5 on the North Side, **and in Span 13 on the South Side**, there is active leakage coming through the utility entrances in the deck. A utility line mounted below the top deck in the south bay is sagging and making contact with the maintenance deck in several locations (*Photo UT-2*). **There is a missing electric junction box cover on the south concrete rail in Span 7 (Photo UT-3)**.

Lighting

The lighting on the bridge is in **Fair** condition. Architectural light pole bases on the north sidewalk in Spans 5, 8 and 11 have cracked and are broken (*Photo UT-4*). **Two of the architectural lights on the north sidewalk, and numerous of the taller, cobra style roadway lights are not functioning**. All the exterior pier shaft light brackets have paint failure and corrosion with minor section loss present (*Photo UT-5*). Many of the architectural lights attached to the lower deck fascia are not functioning, and several are visually broken (*Photo UT-6*).

Recommendations

The General Appraisal and Operating Status for the Veterans Memorial/Detroit Superior Bridge over the Cuyahoga River is in overall **5A, Fair** condition. The superstructure components are the governing element for this condition rating.

The following items are recommended by Palmer Engineering:

Immediate:

- Clean and paint areas of active corrosion on the steel truss.
- Clean and repair clogged and/or damaged drainage system components.
- Repair areas of deteriorated concrete at the arches.
- Repair and clean areas of the deteriorated substructure concrete.
- Determine a permanent repair to correct or stabilize the Tower B rotation.
- Replace the strip seal at the Pier 10 Joint.
- **Repair the broken welds on Joint 1A armor.**
- Repair the navigation lights that are not functioning.
- **Replace missing electric junction box cover on the south concrete rail in Span 7**
- Repair the embankment in Spans 1A and Span 3.
- Secure all access points to the structure, and remove the homeless encampments from Span 1 and Pier 3.

Routine:

- Monitor areas of concrete deterioration that could cause damage to vehicular and pedestrian traffic.
- Clean the expansion joints.
- Look into feasibility of draining the flooded areas of the east and west stations to gain access and perform a thorough inspection.
- Continue to monitor the rotation of the tower shafts.
- Repair non-functioning architectural lighting.

APPENDIX A – INSPECTION PHOTOS

DECK PHOTOS



Photo 12-1 – Detroit Ave Tunnel, Typical Spalling of Non-structural Deck



Photo 12-2 – West Station, Typical Scattered Deck Spalling with Saturation and Efflorescence



Photo 12-3 – Main Spans, Typical Scattered Deck Spalling



Photo 12-4 – Main Spans, Deck Saturation with Mottling and Efflorescence



Photo 12-5 – View of Deck from Maintenance Deck, Span 4, Center Section



Photo 300-1 – Expansion Joint, Typical Debris Impaction



Photo 300-2 – Joint in Span 4 at PP5, Spalling and Cracking of Header at North Sidewalk Expansion Joint



Photo 300-3 – Pier 10 Joint, 4' Section of Seal Tearing



Photo 300-4 Joint 1A, 2' Section of Broken Intermittent Fillet Welds in Joint Armor



Photo 331-1 – South Rail, Typical Cracking and Staining



Photo 331-2 – South Rail Fence, Area of Damage in Span 5



Photo 510-1 – Map Cracking in Wearing Surface



Photo 510-2 – Map Cracking in Wearing Surface



Photo 510-3 – West Station, Areas of Cracking in Wearing Surface



Photo 815-1 – Pier 1, East Face, South Downspout Fully Clogged.



Photo 815-2 – Pier 3, South Downspout Disconnected at Outlet



Photo 815-3 – North Sidewalk Longitudinal Drain, Typical Clogging with Debris



Photo 815-4 – Span 1, South Curb Scupper Inlet Fully Clogged with Debris



Photo 815-5 – Span 13, Heavy Deck Drainage through Utility Manhole



Photo CS-1 – South Sidewalk, Typical Edge Spalling



Photo CS-2 – Span 4 North Sidewalk, Delamination and Cracking along Longitudinal Drain

SUPERSTRUCTURE PHOTOS



Photo 110-1 – West Station, Spalling of Concrete Beam



Photo 144-1 – Span 9 Reinforced Concrete Arches



Photo 144-2 – Span 13, FRP on Underside of Concrete Arches



Photo 144-3 – Span 1, North Exterior Arch, Map Cracking and Delamination of Arch Rib



Photo 144-4 – Span 3, South Exterior Arch, Map Cracking and Delamination of Arch Rib



Photo 144-5 – Span 8, South Interior Arch, Spalling of Arch Column



Photo 144-6 – Span 7, North Interior Arch, Spalling of Arch Column



Photo 144-7 – Span 8, North Exterior Arch, Cracking and Delamination of Jack Arch



Photo 155-1 – Span 3, Lower Deck FBs, South Bay, Cracking, Delamination and Spalling with Exposed Reinforcing



Photo 155-2 – Span 12, Widespread Cracking, Delamination and Spalling of Lower Deck Floorbeams



Photo 155-3 – Span 3, Upper Deck FB 3, Center Bay, Spalling and Delamination



Photo 155-3 – Span 13, Upper Deck FB 6, South Bay, Spalling and Delamination



Photo 155-4 – East Abutment Chamber Floorbeams, Typical Spalling



Photo 155-5 – Span 10, FRP on Underside of Lower Deck Floorbeams



Photo 113-1 – Span 4, Upper Deck Stringers, Typical Surface Corrosion and Minor Pitting on Stringer Ends



Photo 113-2 – Span 4, Upper Deck Stinger E, Bay 4'-5', Active Pitting Corrosion and Painted over Pitting



Photo 113-3 – Span 4, Upper Deck Stinger E, Bay 2-3, 5" L x 2" W Perforation at FB3



Photo 113-4 – Span 4, Lower Deck Stringer K at FB3', Painted over Perforations



Photo 120-1 – Span 4, South Truss, U0-U1, Active Pack Rust between Web Plate and Flange Angles.



Photo 120-1 – Span 4, North Truss, U11-U12, Active Pack Rust between Web Plate and Flange Angles.



Photo 120-2 – Span 4, North Truss, U1'-L2', Active Corrosion extending down from Upper Deck



Photo 120-3 – Span 4, South Truss, L2'-L3", Active Corrosion and Pack Rust



Photo 120-4 – Span 4, South Truss, U1-L1, Painted Over Section Loss and Corrosion Hole in Lacing Bar at Deck Level



Photo 120-5 – Span 4, South Truss, U3-L4,, Perforations in Batten Plate, with Reactivating Corrosion



Photo 120-6 – Span 4, North Truss, L10, Reactivating Pitting around Hanger Pin

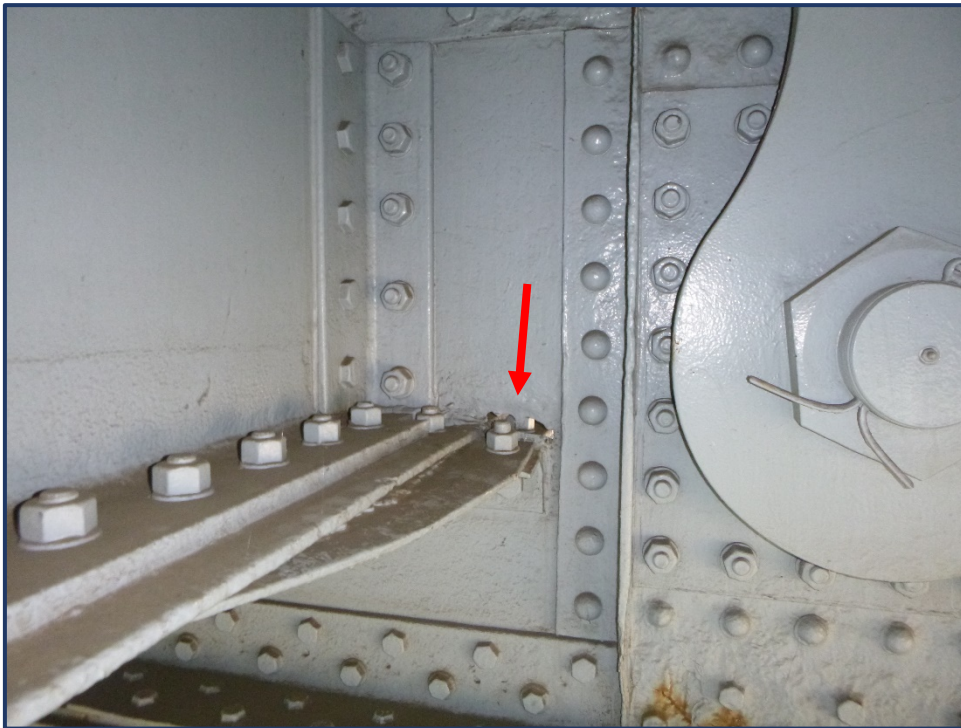


Photo 152-1 – Span 4, Upper FB10 between North Truss & Stringer E, Area of Perforations in Web



Photo 152-2 – Span 4, Upper FB0', Pitting of Floorbeam and Perforations in Stiffeners.



Photo 152-3 – Span 4, Lower FB0, North End, Perforations and Active Corrosion of Bottom Flanges



Photo 152-4 – Span 4, Lower FB9 at South Truss, Pitting and Perforations



Photo 152-5 – Span 4, Lower FB12, North End, West Face, 5-5/8" Crack in Top Flange Stiffener Retrofit



Photo 161-1 – Span 4, North Truss, Hanger Connection at Lower FB4, Pitting around Pin



Photo 162-1 – Span 4, South Truss, L2 South Gusset Plate, Active Pitting along L1-L2



Photo 162-2 – Span 4, South Truss, L2' Gusset Plates, Active Pitting & Pack Rust



Photo BR-1 – Active Corrosion and Pack Rust on Sway Bracing Members

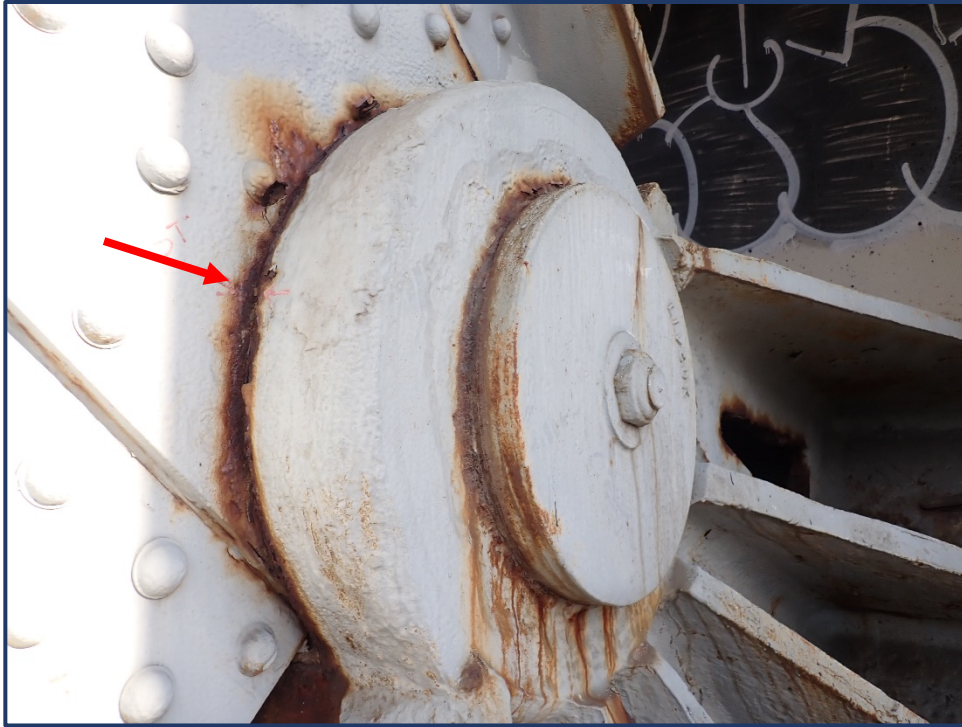


Photo 313-1 – Span 4, South Truss, L0' Bearing, Pack Rust Around Pin

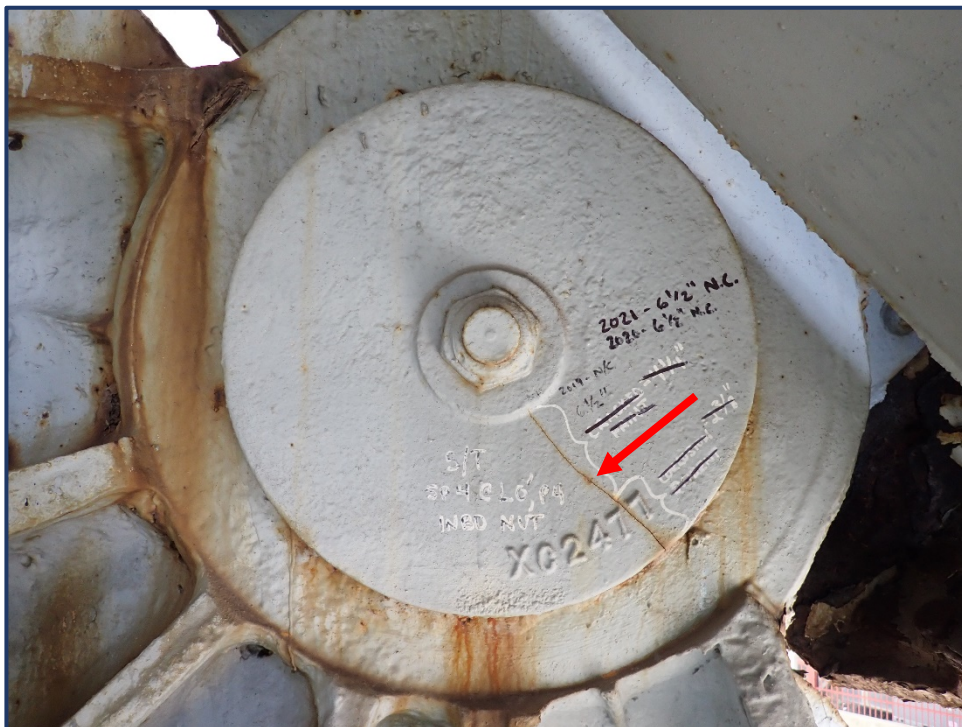


Photo 313-2 – Span 4, South Truss, L0' Bearing, Crack in North Pin Cover Plate



Photo 313-3 – Span 4, North Truss, L0 Bearing, Missing Pin Cover Plate



Photo 313-4 – Span 4, South Truss, L0' Bearing, Anchor Bolt & Nut Section Loss



Photo 313-5 – Pier 11, Slab/Floorbeam Bearing Plates between Ribs C & D, Movement of Bearing Plates



Photo 515-1 – Span 4, Peeling Paint and Surface Corrosion Below Upper Deck

SUBSTRUCTURE PHOTOS



Photo 205-1 – Pier 7, West Face



Photo 205-2 – Pier 11, West Face



Photo 205-3 – Span 1A, Typical Column Spalling with Exposed Reinforcing



Photo 205-4 – West Station, Column Spalling with Exposed Reinforcing



Photo 205-5 – Pier 5, South Tower, Widespread Delamination and Spalling with Exposed Reinforcing



Photo 205-6 – Pier 6, South Tower Leaning Away from Bridge at Top



Photo 210-1 – Pier 4, West Face, Typical Map Cracking in Lower Portion



Photo 210-2 – Pier 3, Southeast Corner, Spalling at Waterline



Photo 210-3 – Pier 4, Typical Spalling of Interior Face of Pier Walls



Photo 215-1 – West Station, Spalling with Exposed Reinforcing on Walls



Photo 215-2 – Detroit Ave. Tunnel, Typical Spalling on Lower Portions of Walls



Photo WW-1 – Span 1A South Wingwall, Area of Spalling with Exposed Reinforcing



Photo WW-2 – West Abutment Chamber, Crack Through Footing at Tower B



Photo AC-1 – West Abutment Chamber, Typical Cracking with Staining and Shallow Spalling

CHANNEL PHOTOS



Photo CH-1 – Downstream Channel, Looking South

APPROACH PHOTOS



Photo APR-1 – Erosion of Embankment on West Side of Pier 3



Photo APR-2 – Erosion Hole and Channel on South Side of Span 1A



Photo APR-3 – Open Fence at Southeast Corner of Pier 1



Photo APR-4 – Hole Cut in Vandal Fence Fabric at South Corner of West Abutment



Photo APR-5 – Hole Cut in Vandal Fence Fabric at South Corner of Span 1A



Photo APR-6 – Failed Preventive Access Steel Mesh on South Side of Span 1A

UTILITY PHOTOS



Photo UT-1 – Deteriorated Lower Deck Utility Junction Chamber Support.



Photo UT-2 – Sagging Utility Line on Maintenance Deck



Photo UT-3 – Missing Electric Junction Box Cover, South Railing, Span 7



Photo UT-4 – Broken Light Pole Base Cover on North Sidewalk



Photo UT-5 – Typical Corrosion and Minor Section Loss on Tower Light Brackets



Photo UT-6 – Broken Architectural Light, Span 8 North Fascia

APPENDIX B – DETAILED INSPECTION FINDING TABLES

Detroit-Superior (Veterans Memorial) Bridge

Table 1: Strip Seal Joint Measurements						
Date:	2021			2020		
	Temperature: 70°F			Temperature: 60°F		
Joint Location	Joint opening measurements			Joint opening measurements		
	Left side	Center	Right Side	Left side	Center	Right Side
Tower A	1-3/4"	3/8"	1-1/4"	1-5/8"	1/4"	1"
Tower B	1-7/8"	1-7/8"	2-1/4"	1-3/4"	1-3/4"	2-1/2"
West Abutment	1-5/8"	1-3/4"	2-1/8"	1-3/4"	1-3/4"	2-1/2"
Pier 1	1-3/8"	1-1/2"	2"	1-7/8"	1-3/4"	2-1/4"
Pier 2	1-1/2"	1-5/8"	1-7/8"	1-3/4"	1-7/8"	2-1/8"
Pier 3 West	1-7/8"	2"	2-1/4"	2-1/8"	2-1/4"	2-1/2"
Pier 3 East	1-1/8"	1-1/8"	1-1/2"	1-1/2"	1-7/8"	1-5/8"
Span 4 at PP5	1-1/2"	1-3/8"	2"	1-1/4"	1-3/8"	2-1/8"
Span 4 at PP5'	1-7/8"	1-5/8"	2"	2-1/8"	1-3/4"	2"
Pier 4 West	1-1/2"	1-1/4"	1-5/8"	1-1/2"	1-3/8"	1-3/4"
Pier 4 East	2-3/8"	2-1/4"	2-3/8"	2-3/8"	2-3/8"	2-1/2"
Pier 5	1-1/2"	1-1/2"	1-7/8"	1-3/4"	1-5/8"	2"
Pier 6	1-3/4"	1-5/8"	2"	2"	1-3/4"	2-1/4"
Pier 7	1-3/4"	1-5/8"	2"	1-7/8"	1-3/4"	2-1/4"
Pier 8	1-7/8"	1-7/8"	2-1/8"	2"	2"	2-1/4"
Pier 9	1-3/4"	1-3/4"	2"	2"	1-7/8"	2-1/8"
Pier 10	1-3/4"	1-7/8"	2-1/8"	1-7/8"	2"	2-1/4"
Pier 11	2"	1-7/8"	2"	2"	2"	2-1/8"
Pier 12	2"	2"	2-3/8"	2-1/8"	2-1/4"	2-1/8"
East Abutment	1-5/8"	1-5/8"	1-3/4"	1-3/4"	1-3/4"	1-3/4"

* Measurements taken between face of armor

Table 2: Top of Deck Deficiencies		
Unit	Span	Defect Description
Sidewalk	1A	South Sidewalk has a 4' L x 4" W x 1" D spall.
Joint	1A	Joint between the west approach and Span 1A is partially filled with debris. The joint is nearly closed in the center and open in the shoulders. There is a 2' long section of broken intermittent field welds on the joint armor in the eastbound lane.
Joint	1A	Joint between Spans 1A and 1B has up to 1/8" D gouges in the north half of the joint armor.
Concrete Railing	1A	South railing at Tower A has 7/8" vertical misalignment.
Concrete Railing	1B	South railing at Tower B has 1/2" vertical misalignment.
Drainage	1	South shoulder near the West Abutment has a clogged scupper.
Sidewalk	3	8' L x 9" W crack and delamination in the south sidewalk near Pier 3 West.
Sidewalk	4	2' L x 14" W x 2" D spall in the north sidewalk at the Pier 3 west joint.
Curb	4	Random spalling along the curbs, full length of span.
Concrete Railing	4	North and south concrete railing have spalls up to 2'-0" H x 9" W x 4" D along the joint where the utility conduits are exposed.
Sidewalk	4	FW x up to 12" L spalling and delaminated concrete on both sides on the North Sidewalk joint header at the Pier 3 east joint.
Sidewalk	4	North Sidewalk has intermittent spalling over FL x 18" W x up to 1- 1/2" D with cracking and delamination typical along the longitudinal drain. Longitudinal drains are fully clogged.
Sidewalk	4	Intermittent spalling and cracking along either side of joint at Panel Point 5' in the north sidewalk up to 1/2" D.
Sidewalk	4	Intermittent spalling along either side of joint at Panel Point 5 in the north sidewalk up to 1/2" D.
Sidewalk	4	North sidewalk has intermittent spalling over a 50' L x 18" W x up to 1-1/2" D.
Sidewalk	4	30' L x 2" D spall around the longitudinal drain on the north sidewalk near the pier 3 east joint.
Curb	4	Typical spalling in Right Truss curb up to 3" D.
Sidewalk	4	5' L x 6" W x 2" D spall in South Sidewalk at Panel Point 2.
Sidewalk	4	2'-7" L x 6" W x up to 2" D spall in South Sidewalk at Panel Point 5.
Sidewalk	4	2'-4" L x 8" W x 2" D spall in South Curb at Panel Point 8.
Sidewalk	4	2'-5" L x 6" W x 1-1/2" D spall in South Sidewalk at Panel Point 9.
Sidewalk	4	2' L x 7" W x 2" D spall in South Sidewalk at Panel Point 10.
Sidewalk	4	2' L x 19" W x 2" D spall in South Sidewalk at Panel Point 10'.
Sidewalk	4	7' L x Full Width Delamination on South Sidewalk at Panel Point 4'.
Sidewalk	4	2'-8" L x 7" W x 1-1/2" D spall in South Sidewalk at Panel Point 1'.
Sidewalk	4	2'-4" L x 6" W x 2" D spall in South Sidewalk at Panel Point 1'.
Lighting	4	Base of light pole is broken at Pier 4 west.
Lighting	Pier 4	Base of light pole on north sidewalk over Pier 4 is broken.
Sidewalk	Pier 4	3'-0" L x 13" W x 1-1/2" D spall in the north sidewalk near the pier 4 east joint.
Vandal Protection Fence	5	South vandal protection fence, top of rail is bent outward 2' L.

Table 2: Top of Deck Deficiencies		
Unit	Span	Defect Description
Sidewalk	6	5' L x 13" W x 2" D spall in the North sidewalk.
Sidewalk	7	2' L x 8" W - 1-1/2" D spall in South Sidewalk.
Sidewalk	7	2'-4" L x 6" W x 1-1/2" D spall in South Sidewalk.
Sidewalk	7	15' L x 25' W delamination in the South Sidewalk.
Vandal Protection Fence	7	South vandal protection fence has damage over a 4' Length.
Lighting	7	Missing pullbox cover on the South side.
Drainage	7	There is a missing drain cover on the longitudinal drain near Pier 6.
Sidewalk	Pier 7	15' L x 4' W delamination with edge spalling in the south sidewalk.
Vandal Protection Fence	8	South vandal protection fence has collision damage over a 6' L section (entire single section) with one cracked/broken main vertical.
Lighting	8	Decorative base of light pole on north sidewalk is broken.
Curb	9	2'-8" L x 7" W x 1-1/2" D spall in south sidewalk at top of curb.
Joint	10	4' L section of torn joint seal in EB Lanes
Vandal Protection Fence	10	South vandal protection fence has impact damage over a 2' Length.
Lighting	11	Base of light pole on north sidewalk is broken.
Sidewalk	12	2'-9" L x 6" W x 2"D spall in the south sidewalk.
Longitudinal Drains	All	Longitudinal Drains in North Sidewalk are fully are partially clogged almost entire length.
Concrete Railing	All	Scattered vertical cracks with leakage.
Joint	All	Debris is in the joints for 50% of their length.
Joint	west appr.	Full width crack in the approach end of the joint armor at the roadway centerline.

TABLE 3: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR NORTH TRUSS			
Member Type	Upper/ Lower Deck	Member	Deficiency
FB	UD	FB0	Painted over section loss to the north knee brace for the upper deck floorbeam.
FB	LD	FB0	There are large perforations and areas of up to 100% section loss on the top and bottom angles on the west and east faces of the north end of the floorbeam cantilever. There are areas of active corrosion and up to 1/8" D pitting on the floorbeam web at this location on the west face of the floorbeam cantilever web. There are corrosion holes in the transverse stiffeners at the north cantilever measuring 6" H x Full Width. The rivets and bolts have 50-75% section loss at the north end of the floorbeam cantilever.
FB	LD	FB1	Painted over 10% section loss to the top and bottom east flanges of the lower deck floorbeam.
Stringer	UD	PP2-3	Numerous corrosion holes in the bottom north flange of Stringer "E".
Stringer	UD	PP2-3	5" L x 2" W corrosion hole on the bottom flange of Stringer "E" near FB3.
FB	LD	FB3	End 3' L on both sides have up to 5/16" D painted over pitting on the webs and bottom flanges with several perforations on the ends of the bottom flanges.
Eyebar	LD	FB4	Bottom head of east eye bar has up to 3/16" D painted over pitting around the pin. Stiffening plates on floorbeam web behind the eyebar head have 18" H x 20" W corrosion holes on the west and east faces.
FB	UD	FB10	There is a 4" W x 1-1/8" H hole in the floorbeam web north of Stringer "E".
FB	UD	FB10	There is a 8" W x 1.5" H hole in the floorbeam web south of Stringer "E".
FB	LD	FB10	8.5" L x 1-1/2" W crack along the weld of stiffening plate welded to floorbeam top flange, east face of floorbeam at L10.
FB	UD	FB12	7-1/4" W x 4" H corrosion hole in the floorbeam web north of Stringer "E".
FB	UD	FB12	9" W x 1" H hole in the floorbeam web south of Stringer "E".
FB	LD	FB12	5-5/8" L crack on the top flange stiffener plate on the west face.
FB	UD	FB11'	3-1/2" W x 2-1/2" H hole in the floorbeam web north of Stringer "E".
FB	UD	FB11'	1" W x 2" H hole in the floorbeam web south of Stringer "E".
FB	LD	FB11'	4-3/4" crack along weld of stiffening plate welded to floorbeam top flange, west face of floorbeam.
FB	UD	FB10'	There is a 7" W x 2-1/4" H and a 4-1/4" diameter corrosion hole in the floorbeam web north of Stringer "E".
FB	UD	FB10'	There is a 7" W x 1-1/4" H hole in the floorbeam web south of Stringer "E".
FB	LD	FB10'	2" long longitudinal crack and a 1- 7/8" long transverse crack along weld of stiffening plate welded to floorbeam top flange, west face of floorbeam.
FB	LD	FB9'	3" crack on the top flange stiffener plate on the west face.
FB	UD	FB8'	5-1/2" W x 11-1/2" H corrosion hole in the floorbeam web north of the first interior stringer from the panel point with adjacent advanced painted over pitting.
FB	UD	FB8'	20" W x 2" H corrosion hole in the floorbeam web south of the first interior stringer from the north truss. 1-3/4" tear in the stiffener between stringers 6-7 due to construction damage.

TABLE 3: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR NORTH TRUSS

Member Type	Upper/ Lower Deck	Member	Deficiency
FB	LD	FB8'	3" crack in the top flange stiffener plate weld on the west face of the floorbeam adjacent to the end of the north knee brace due to section loss and corrosion. There is up to 5/16" D painted over section loss to the floorbeam web and flanges near the north truss. There is one 1-1/2" Dia. hole in the west bottom flange below the first interior stringer, and a 3" H x Full Width hole in the bottom of the transverse stiffener. South of the north truss, there are 7 rivet heads that have up to 100% section loss between the west bottom flange and the bottom flange cover plate.
FB	LD	FB7'	2' H X 2' W area of average 1/4" D painted pitting with two corrosion holes in the web of the floorbeam between the strut and stringer, west face. There are three 1" Dia, perforations in the west bottom flange between the north truss and the first interior stringer. The west face of the bottom flange cover plate near the north truss has 4 rivet heads with 100% section loss, and 10 rivet heads with 50-75% section loss.
FB	LD	FB7'	There are several cracks in the welds between the East top flange of the floorbeam and the welded retrofit angle: 4-1/4", 5", and 1-3/4" cracks along the east edge; 2-5/8" crack along the north edge. These cracks are due to advanced section loss of the retrofit angle. Up to 1/4" D pitting in the top of the east bottom flanges at the north truss.
FB	UD	FB6'	Up to 1/8" D painted over pitting on the west eyebar face around the lower portion of the pin. There is a 6-1/2" L x 1-1/2" H corrosion hole in the floorbeam web north of the first interior stringer from the panel point.
FB	LD	FB6'	The floorbeam has active corrosion and 1/16" D pitting on the top and bottom flanges between the north and south trusses. There are welded repair plates on the web at the north truss. On one of these, there is a 10" L cracked perimeter along the weld to the floorbeam top flange, west face of floorbeam due to 100% section loss of the repair plate. 2-11/16" L paint crack in the vertical weld to the knee brace. At the north truss, the web and flanges have 1/8" to 3/16" D painted over pitting. The north cantilever is in a similar condition and has isolated areas of active corrosion.
FB	UD	FB4'	Painted over corrosion holes (up to 3" Dia.) in the web just south of the north truss with painted over pitting below the upper deck lateral bracing gusset plate.

TABLE 3: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR NORTH TRUSS

Member Type	Upper/ Lower Deck	Member	Deficiency
FB	LD	FB4'	The floorbeam web has painted over pitting up to 1/4" D, worst around the stringers, connection, and lateral bracing, with isolated pitting on the welded repair plate. The west eyebar head has painted over pitting 1/8" to 3/16" D for 6" L around the nut. Additional web plates behind the eyebar have up to 100% section loss in the bottom flange. The north cantilever has small areas of 100% section loss in the bottom flange and transverse stiffener base. Isolated areas of active corrosion with minor laminated pack rust in the bottom flanges and web along the bottom flanges. The west bottom flange is missing for 15" L on the south side of the truss with up to 90% loss of rivet heads in O.L. (?) between the truss and center of the deck. The east eyebar head has isolated painted over pitting up to 1/16" D. The pin behind the eyebar has laminated pack rust up to 3/16" T with associated pitting 1/16" to 1/8" D (some active). The web plates behind the pin have up to 3/8" painted over pitting with 100% section loss. The east bottom flange on the south side of the truss is missing for 24" L. There are numerous painted over corrosion holes within the bottom flange of the floorbeam adjacent to the sway bracing connection. Areas of reactivated surface corrosion were noted throughout the top and bottom flanges. The sway bracing on the east side also has reactivated surface corrosion in the end 6'.
SWAY	LD	LD3'	The majority of the original steel components at the lower deck connection above L3' have areas of painted over pitting with corrosion holes and knife edging.
FB	LD	FB3'	The north end of the floorbeam cantilever web and bottom flanges have up to 5/16" D pitting with a 1/4" Dia. perforation in the web and a 5" L x 1" W perforation in the west bottom flange. In the north catwalk framing, the south stringer at the west face of FB 3' has a 4" L x 3" W perforation in the web and a 8" L x 3" W perforation in the bottom flange.
FB	LD	FB2'	There is 5/16" D painted over pitting with a 1" Dia. hole in the west face of the floorbeam web just south of the north truss along the stringer. There is 100% section loss to the top flange angles over 2' L area with loss of 7 consecutive rivet heads in the same location. Bolts for the lateral bracing connection have up to 50% section loss. At the north cantilever: The bottom and top flanges on both faces have isolated corrosion holes up to 6" L; The transverse stiffener on the west bottom flange has a 4" Dia. corrosion hole; The webs have up to 1/4" D painted over section loss around connections and along horizontal surfaces; The webs have welded repair plates below the walkway; Internal diaphragms between knee braces have isolated 100% section loss and the horizontal diaphragm is nearly gone; Sidewalk stringer has areas of 100% section loss at the connection to the floorbeam.
FB	LD	FB1'	At the north cantilever: There is a 1" Dia. corrosion hole in the north end of the bottom flange with up to 3/16" D pitting throughout the bottom flange and the bottom of the web, and up to 75% section loss to 3 rivets; The cantilever end plate has up to 1" Dia. perforations with pitting; The bottom batten plate at the north end has pack rust, pitting, and perforations. At the Level 3 Strut: The north connection plate for the Level 3 horizontal strut has a 6" x 6" perforation with multiple other perforations at the L1'-U1' connection. At the north catwalk: The south bottom bracing connection plate for the catwalk has a 10" L x 1" W perforation.

TABLE 3: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR NORTH TRUSS

Member Type	Upper/ Lower Deck	Member	Deficiency
FB	UD	FBO'	The upper deck floorbeam has typically up to 1/8" D painted over pitting throughout the bottom flange angles and cover plates near the north truss line and throughout the north cantilever. Several transverse stiffeners on the east face of the floorbeam have up to 6" H x Full Width areas of 100% painted over section loss.
FB	LD	FBO'	The lower deck floorbeam has typical painted over pitting up to 1/8" D. At the north canilever: There is 100% section loss to the west bottom flange over a 6' length with some reactivating corrosion; The west floorbeam top flange adjacent to the LO'N - UO'N vertical has up to 100% painted over section loss for a 3' length; The east floorbeam bottom flange has up to 100% section loss over an 8' length with active corrosion; The north end of the web has multiple perforations (one 4" Dia. and numerous 1/2" Dia. holes) in the web; One high-strength bolt is missing in the top flange. At the north Level 3 strut: Active corrosion with areas of 100% section loss up to 4" L x 2" W area present in the angles and lacing bars; The north vertical connection plate at LO'-U1' has 100% section loss on top of the strut. Below the center deck (?), there is painted over pitting with negligible section loss on the west face.

TABLE 4: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR SOUTH TRUSS			
Member Type	Upper/ Lower Deck	Member	Deficiency
FB	UD	TYP.	The transverse stiffeners on the north and south sides of the eyebars have 100% section loss at the bottom.
FB	UD	U0	Exterior and interior west knee brace has an 18" L x up to 4" W area of painted over corrosion holes. The west web plates also have up to 18" H x 12" W L-shaped painted over corrosion holes in the upper corners.
Stringer	UD	FB0	2" L x 1" W corrosion hole
FB	LD	FB0	End 3' of the web has painted over corrosion holes.
FB	LD	L2	The south diagonal floorbeam support has four 1-1/2" D holes over a 6" D area in the web near the L2 connection. Bottom flanges have areas of 100% section loss x Full W x 20" L. Internal diaphragm plate between lateral bracing connection plates has widespread corrosion holes (nearly gone.)
Stringer	LD	FB3	Second stringer from the south, east face of LDFB3 has a 12" L x 3" H area of painted over pitting with a 6" L x 3" H corrosion hole in the bottom of the web. The north bottom flange has a 10" L x 4" W corrosion hole at the end.
FB	LD	FB3	The east face bottom flange at the L3 connection has a 10" L x 4" W hole.
Eyebar	LD	LD4	UD4-LD4 east eyebar has up to 1/4" D painted over pitting along the south half of the pin nut for 2" W.
FB	LD	FB8	The east bottom flange has a 6" x 6" corrosion hole on the south side of the south truss with up to 100% section loss on 8 adjacent bottom flange vertical leg rivet heads. There are small holes in the ends of the knee braces over the floorbeam.
Hanger	LD	LD9	LD9 has painted over corrosion holes above the floorbeam top flange. There is advanced section loss with a 1/2" Dia. hole in the web. The east bottom flange is gone for 15" L on each side of the truss. The west bottom flange has a 4" Dia. hole in the south side.
FB	LD	FB10	There is a 1" L x 1/2" H corrosion hole in the web on the north side of the south truss. There are corrosion holes up to 6" L at the ends of the knee braces over the floorbeam. There is a 15" L x 2" W corrosion hole along the edge of the east top flange south of the south truss.
Stringer	LD	LD10	Second stringer from the south, west and east face of FB10 has a 12" L x 2" W painted over hole at the end of both south and north top flanges.
Vertical Hanger	LD	LD12	LD12 has painted over corrosion holes between the flanges and 4' below the upper floorbeam bottom flanges/
FB	LD	FB11	There is a 3" downward impact deformation on the east face of the longitudinal stiffener gusset plate. The east bottom flange is gone for 30" L on the north side of the south truss, and an 8" L x 3" W portion is gone on the south side of the truss. The west bottom flange has several corrosion holes up to 3" x 1-1/2" on the south side of the truss.

TABLE 4: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR SOUTH TRUSS			
Member Type	Upper/ Lower Deck	Member	Deficiency
Stringer	LD	LD11	Second stringer from the south, west face of FB11 has a 12" L x 3" W painted over corrosion hole at the end of both south and north top flanges.
Stringer	LD	LD12	Second stringer from the south, west and east face of FB12 has a 12" L x 1" W painted over corrosion hole at the end of both south and north top flanges.
FB	LD	FB11'	The east bottom flanges are gone on the north (30" L) and south (12" L) sides of the south truss. The east top flange has several corrosion holes up to 4" Dia. over a 15" length on the south side of the south truss. There is up to 90% rivet head loss in the top flange cover plate.
FB	UD	FB11'	At the east face, there is a 2" Dia. corrosion hole in the transverse stiffener on the south side of the eyebar near the top.
Stringer	LD	LD11'	Second stringer from the south, west face of FB11' has a 12" L x 3" W painted over corrosion hole at the end of both south and north top flanges.
Stringer	LD	LD11'	The south stringer, east face of FB11' has a 12" L x 2" H area of pitting with a 2" Diameter corrosion hole on the web
FB	LD	FB10'	The east bottom flange on the south side of the south truss has several 1/2" Dia. corrosion holes. The web has painted over pitting up to 1/4" D neat the truss. Numerous rivet heads on the west bottom flange on the north side of the south truss have up to 75% section loss.
Stringer	LD	LD10'	Second stringer from the south at FB10' on the east face has a 12" L x 2" W painted over corrosion hole at the end of both south and north top flanges.
FB	LD	FB9'	The west bottom flange on the north side of the south truss is gone for 20" L. The adjacent transverse stiffener has 100% section loss for the bottom 6".
Stringer	LD	FB8'	Second stringer from the south, west face of FB8' has a 6" L x 3" W painted over corrosion hole on the south top flange. The east face of FB8' has a 1" L x 1/2" W painted over corrosion hole on the north top flange.
FB	LD	FB8'	There are two 1" Dia. corrosion holes in the floorbeam web on the north side of the south truss. The web and flanges have isolated painted over pitting (typ. up to 3/16" D) near the south truss.
FB	LD	FB7'	There is a 14" long section of the west bottom flange that is missing in the south cantilever. A 5" L x 1" H corrosion hole is present in the floorbeam web of the north side of the south truss. There is 100% section loss at the base of the stiffener below the adjacent stringer. There is a 10" L x 8" W hole in the top flange cover plate.
Stringer	LD	FB7'	Second stringer from the south, west and east face of FB7' has 5" L x 1" W painted over corrosion holes on both north and south top flanges.
FB	LD	FB6'	There is a 1" Dia. corrosion hole in the web on the north side of the south truss, and isolated painted over pitting up to 1/8" D.
Eyebar	LD	UD5'	The eyebar heads have painted over pitting around the full perimeter of the pin nut.
FB	LD	FB3'	There are multiple corrosion holes up to 6" Dia. in the east bottom flange of the south cantilever.

TABLE 4: FLOORBEAM DEFICIENCIES IN UPPER & LOWER DECK NEAR SOUTH TRUSS			
Member Type	Upper/ Lower Deck	Member	Deficiency
Stringer	LD	LD3'	The stringer to floorbeam bearing on the west side of the floorbeam has painted over pack rust, missing anchor bolt and corrosion holes. This note is typical at the south truss. The stringer adjacent to the south truss vibrates under live load due to 100% section loss of the bottom flange. There is no connection of the stringer to the bearing assembly. The lower floorbeam bottom flange below has a 6" corrosion hole.
Bracing	LD	LD3'	Typical corrosion holes along the bottom 6" portions of vertical bracing connections to floorbeam flanges at the vertical connection
Sway Bracing	LD	LD3'	The majority of the original steel components at the lower deck connection above L3' have areas of painted over pitting with corrosion holes and knife edging.
FB	LD	FB2'	The web has 3/16" D painted over pitting around the strut connection, and isolated corrosion holes up to 6" L x 4" W in the bottom flanges.
FB	UD	FB1'	The upper knee braces at the vertical connection have 100% section loss and/or knife-edging in the webs and flanges.
FB	UD	FB0'	There is a 1/4" painted over section loss with one 3/4" Dia. corrosion hole north of the south truss in the web between the lateral bracing connection plates.
FB	UD	FB0'	Transverse stiffeners have widespread corrosion holes and section loss north of the south truss on the east face. The upper knee braces at the vertical connection have 100% section loss and/or knife-edging in the webs and flanges.

Table 5: Upper Pier Shaft Rotation Measurements										
Date	Location									
	5*		5 at Lower Deck**		6*		6 at Lower Deck**		7*	
	East Side	West Side	East Side	West Side	East Side	West Side	East Side	West Side	East Side	West Side
2018	14-11/16"	15-1/8"			16"	16"			15"	14-13/16"
2019	14-11/16"	15-1/8"	0"	0"	16-1/8"	16-1/8"	13/16"	1"	15"	14-13/16"
2020	14-11/16"	15-1/8"	0"	0"	16-1/8"	16-1/8"	13/16"	1"	15"	14-13/16"
2021	14-11/16"	15-1/8"	0"	0"	16-1/8"	16-1/4"	3/4"	1"	15"	14-13/16"

* Measurements taken 3' above approximate top of strut wall.

** Measurements taken 6" from east and west edges of the shafts.

Table 6: Crack Gage Measurements - Tower B South

Date	No. 5B Tower B at Lower Deck		No. 1		No. 2		No. 3	
	V (mm)	H (mm)	V (mm)	H (mm)	V (mm)	H (mm)	V (mm)	H (mm)
5/17/07	---	---						
4/16/13	---	---						
10/3/14	6.0	2.0						
8/16/15	8.0	2.0						
9/14/16	11.0	2.0						
11/27/17								
10/22/18								
11/18/19					4.8	2.0		
10/22/20			4.0	1.0	5.0	1.0	0.5	5.0
10/29/21			4.8	1.2	6.0	0.3	0.5	7.0

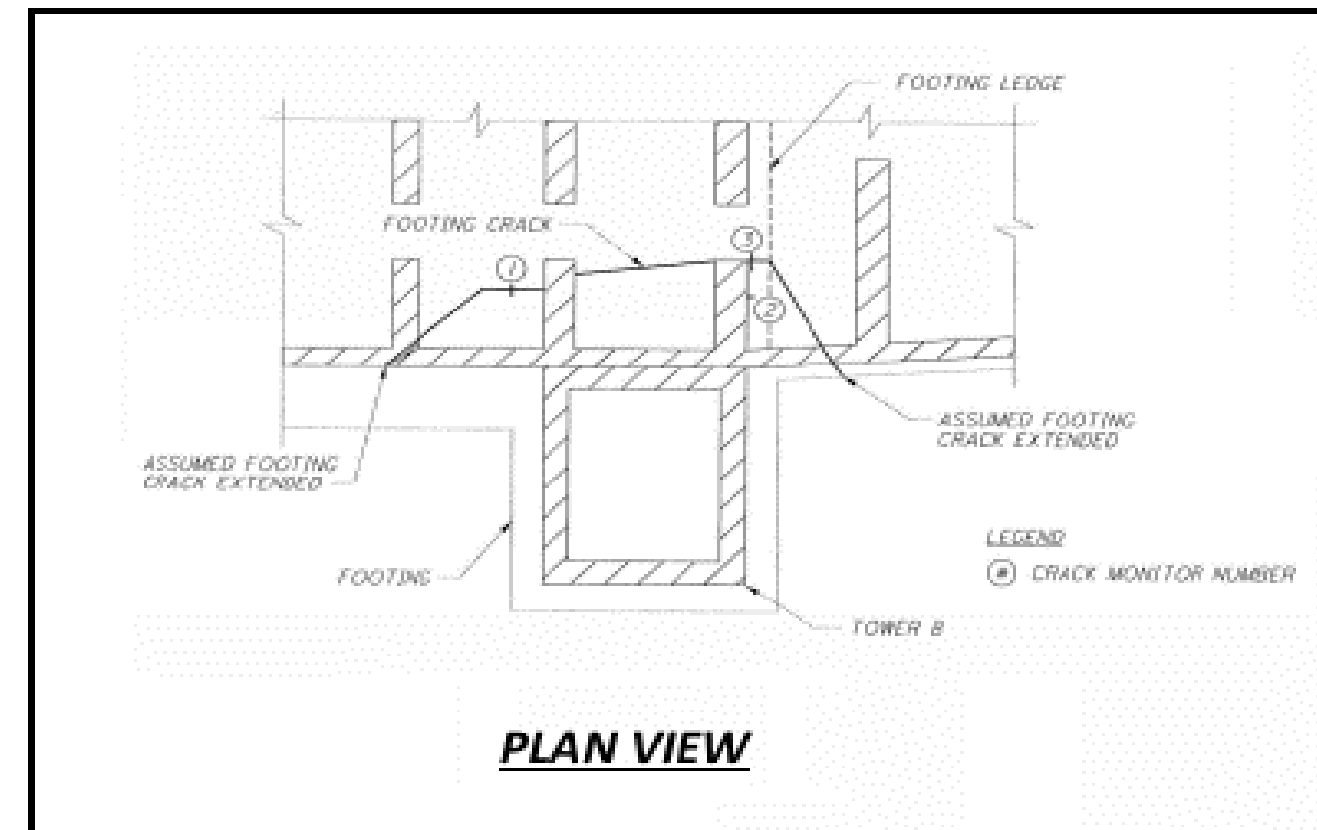
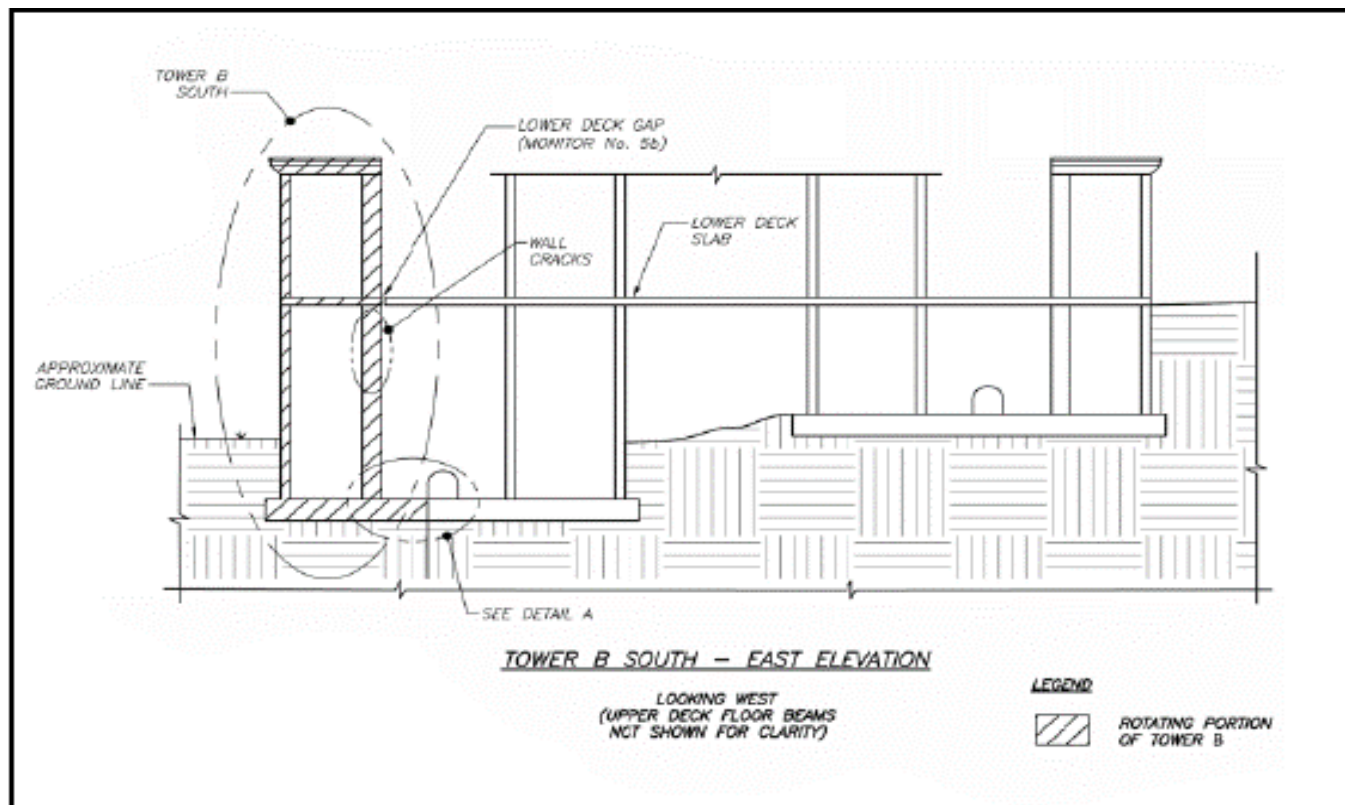
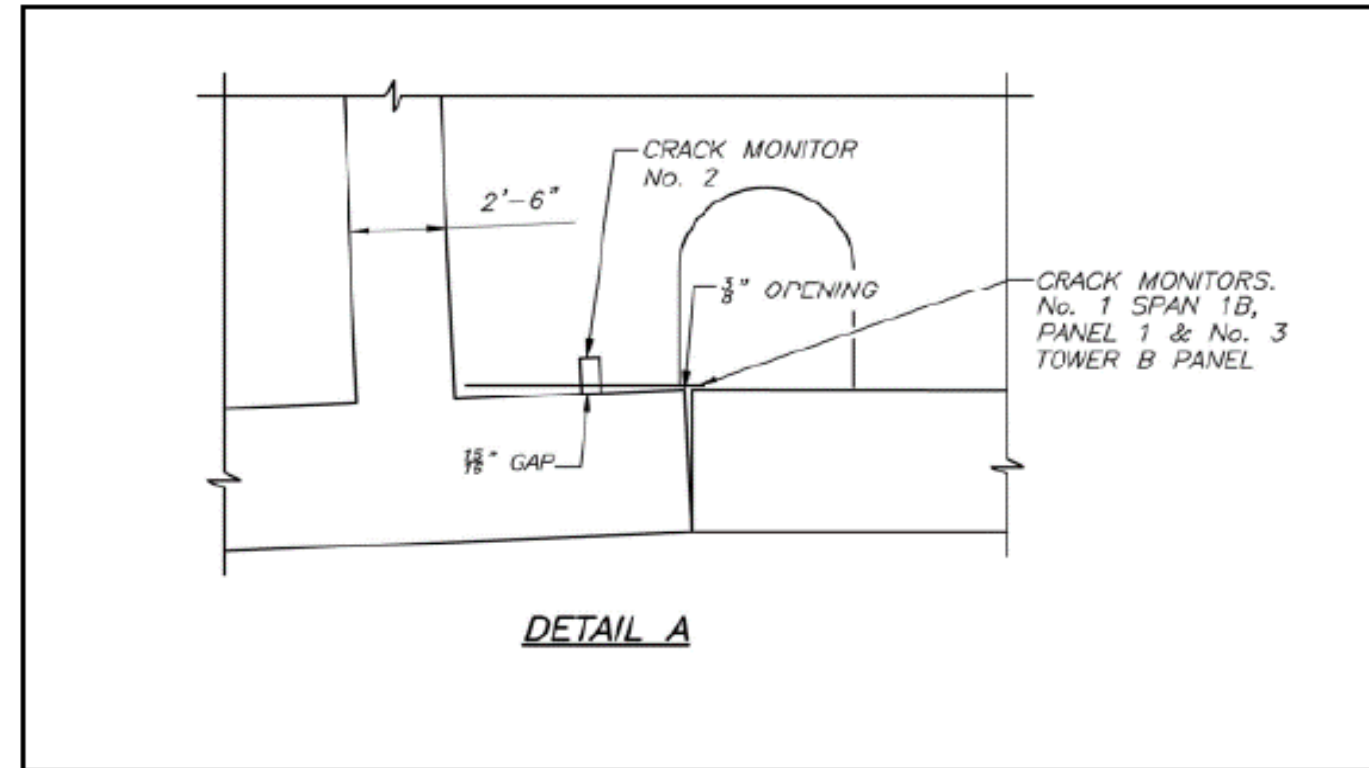


Table 6: Crack Gage Measurements - Span 1A Cellular Construction South Wall

Date	No. 1		No. 2		No. 3		No. 4	
	V (mm)	H (mm)	V (mm)	H (mm)	V (mm)	H (mm)	V (mm)	H (mm)
5/17/07	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4/16/13	2.0	1.0	2.5	0.3	0.0	0.0	0.8	0.2
10/3/14	2.3	1.0	3.0	0.6	0.0	0.0	0.8	0.2
8/16/15	2.8	1.1	4.5	0.8	0.0	0.0	0.9	0.5
9/14/16	3.0	1.1	4.8	0.8	0.0	0.0	1.0	0.5
11/27/17								
10/22/18							1.1	1.0
11/18/19							1.3	1.0
10/22/20					0.0	0.0	1.5	1.0
10/29/21	6.0	1.3	6.0	1.0	0.0	0.00	1.5	1.0

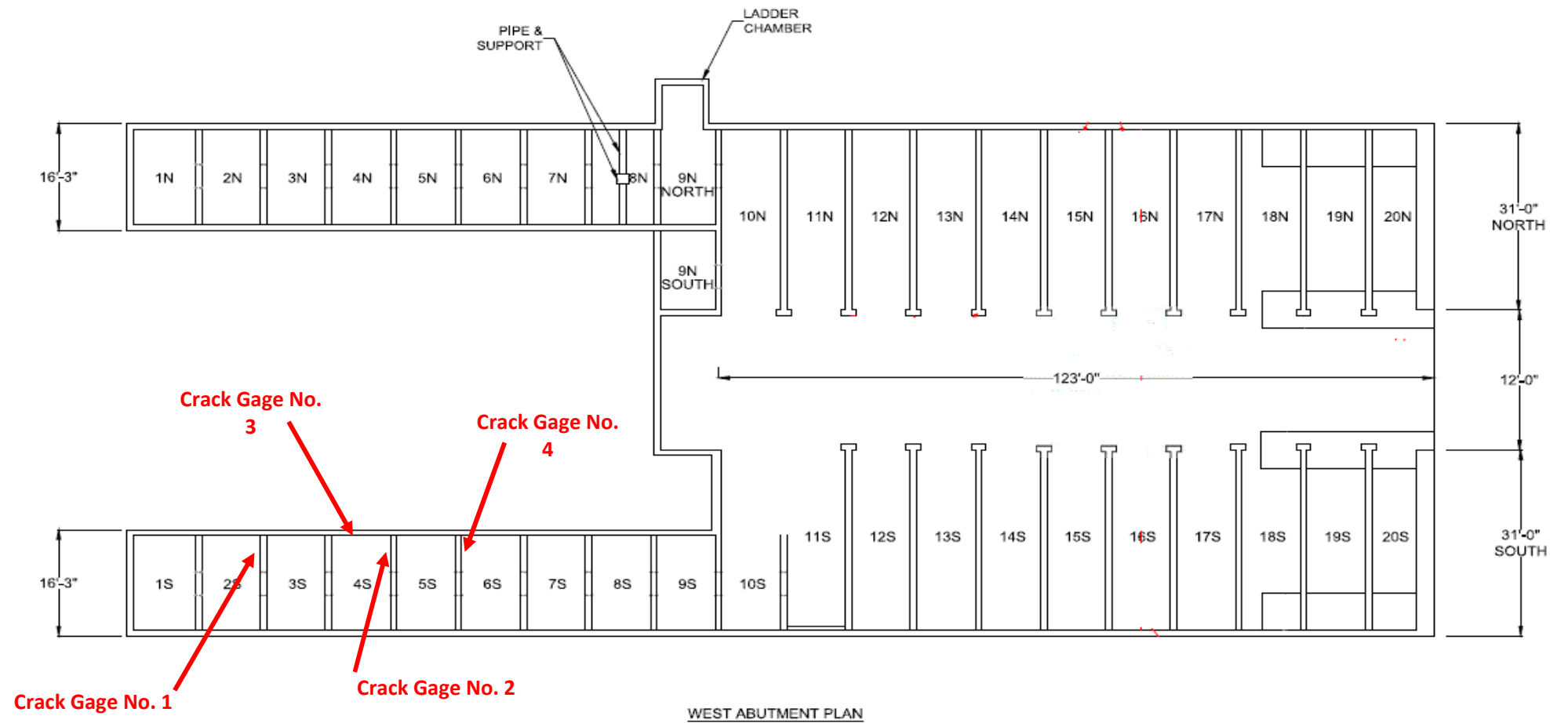
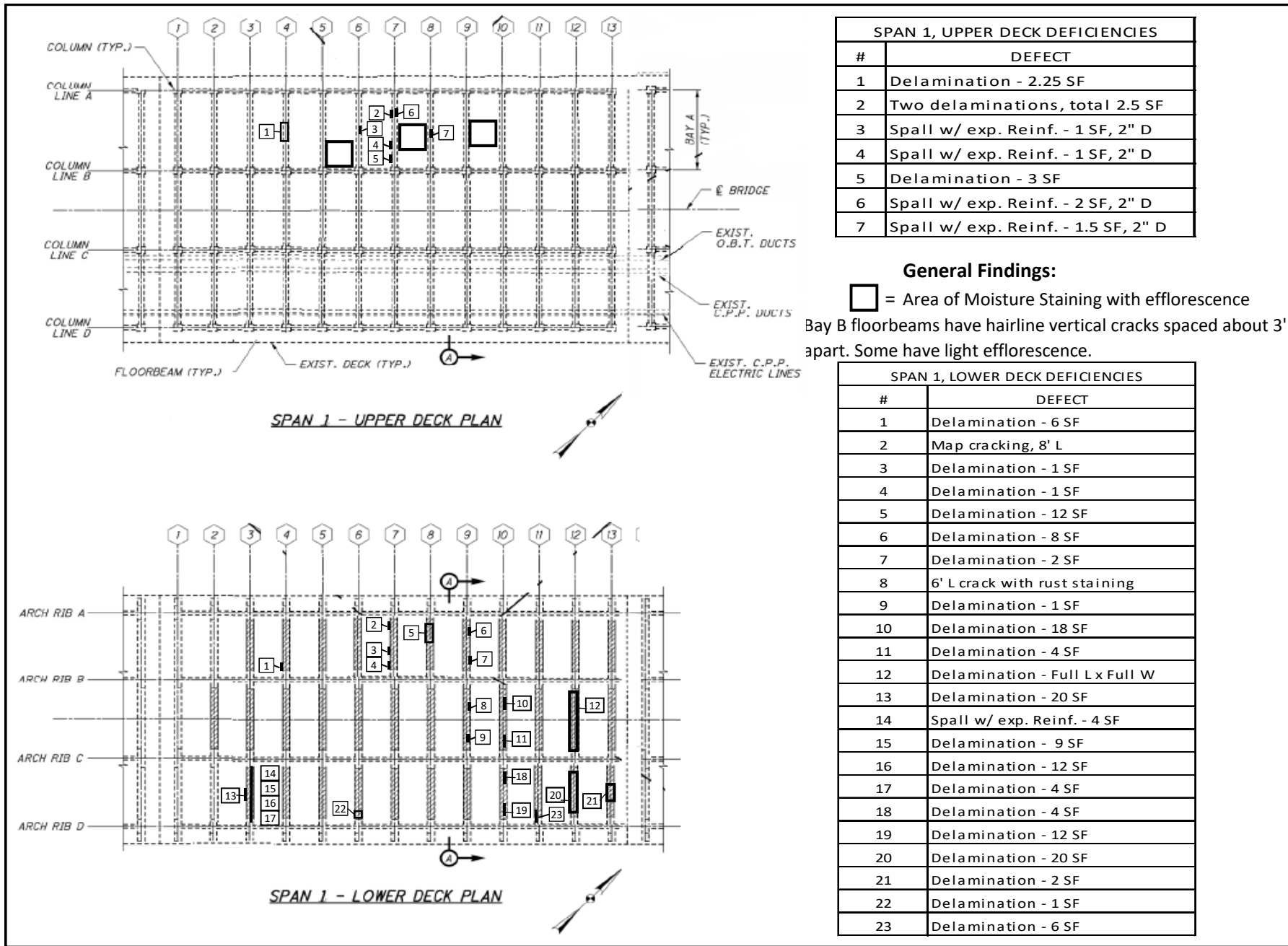
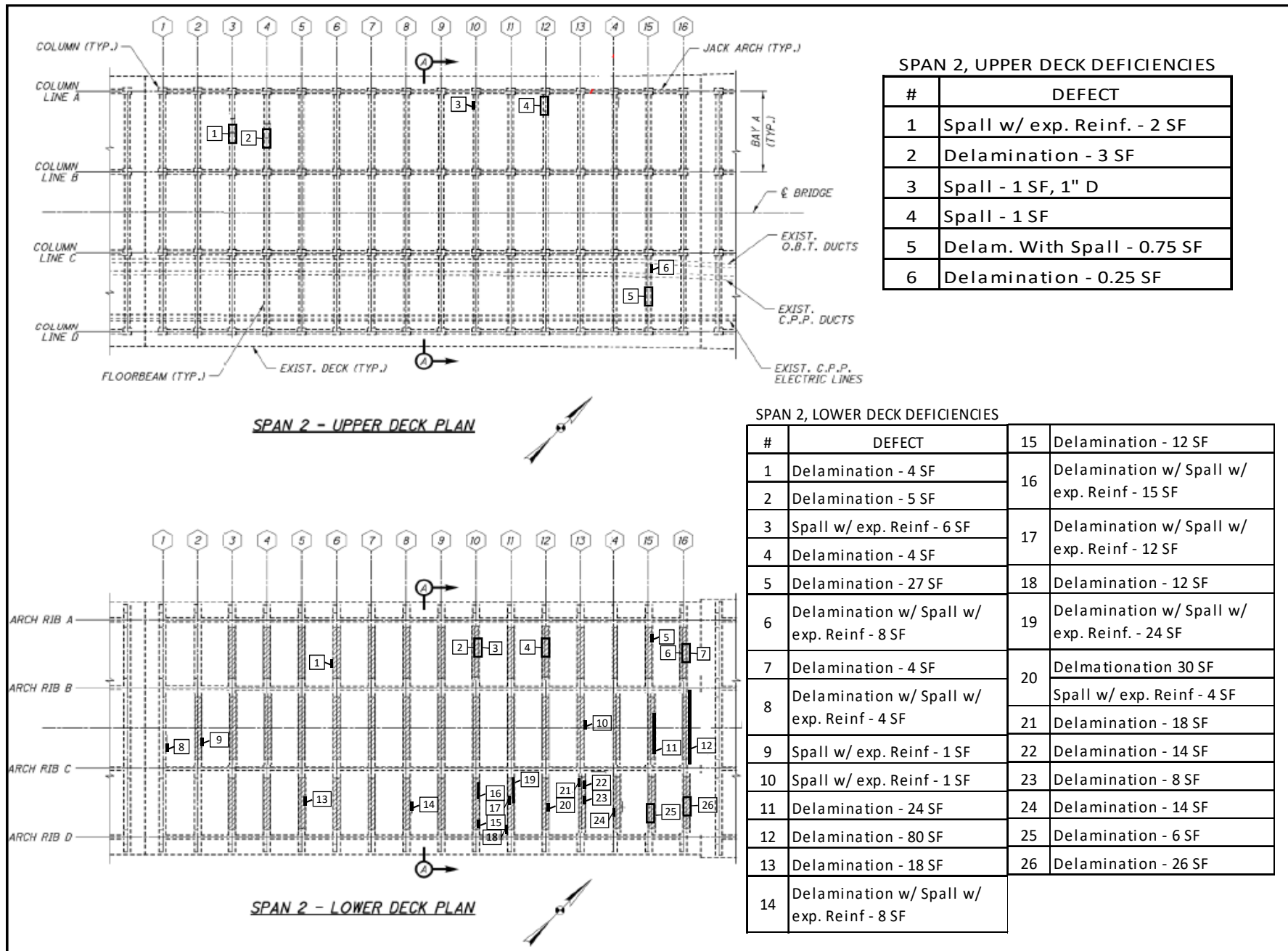
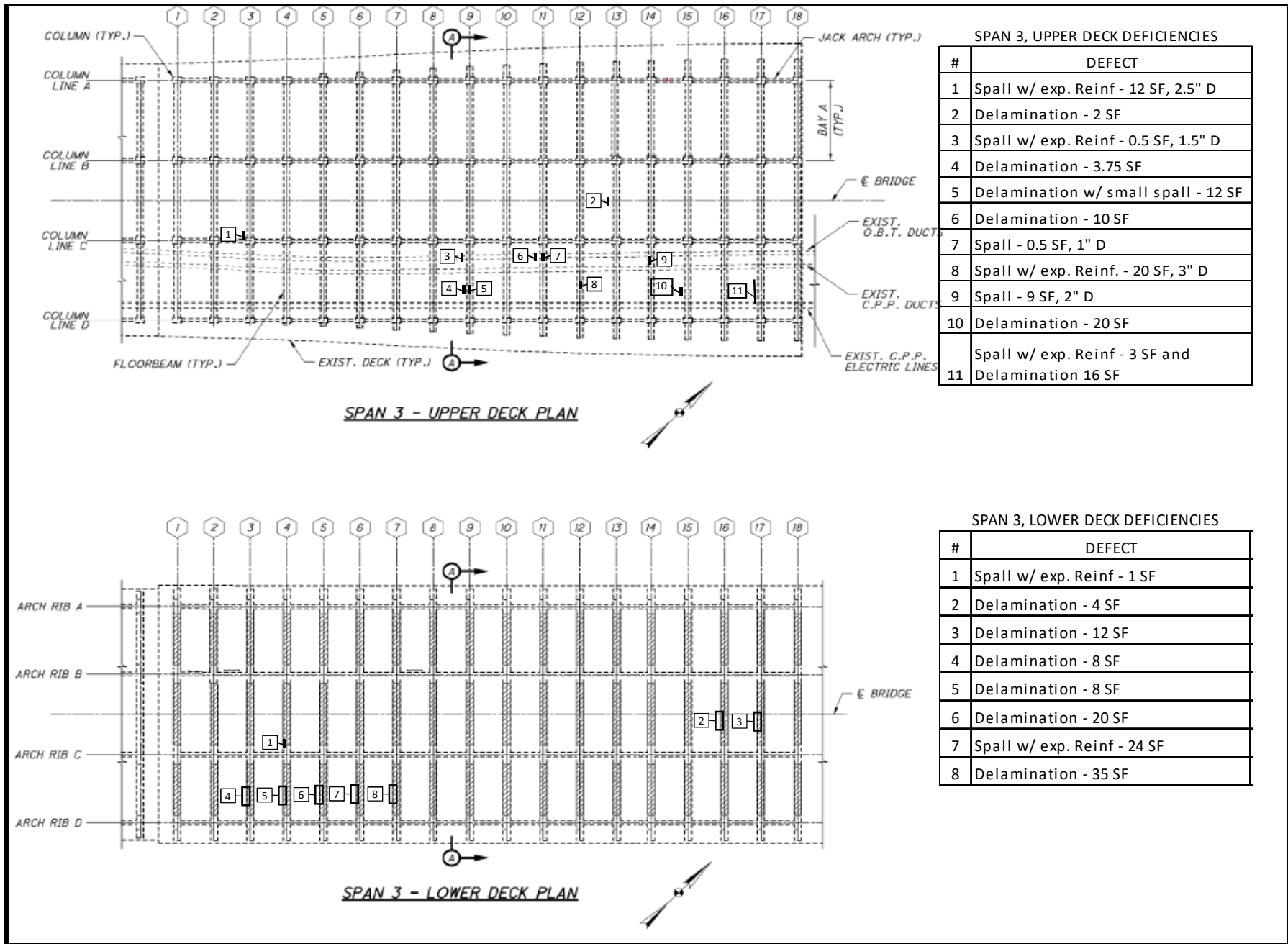


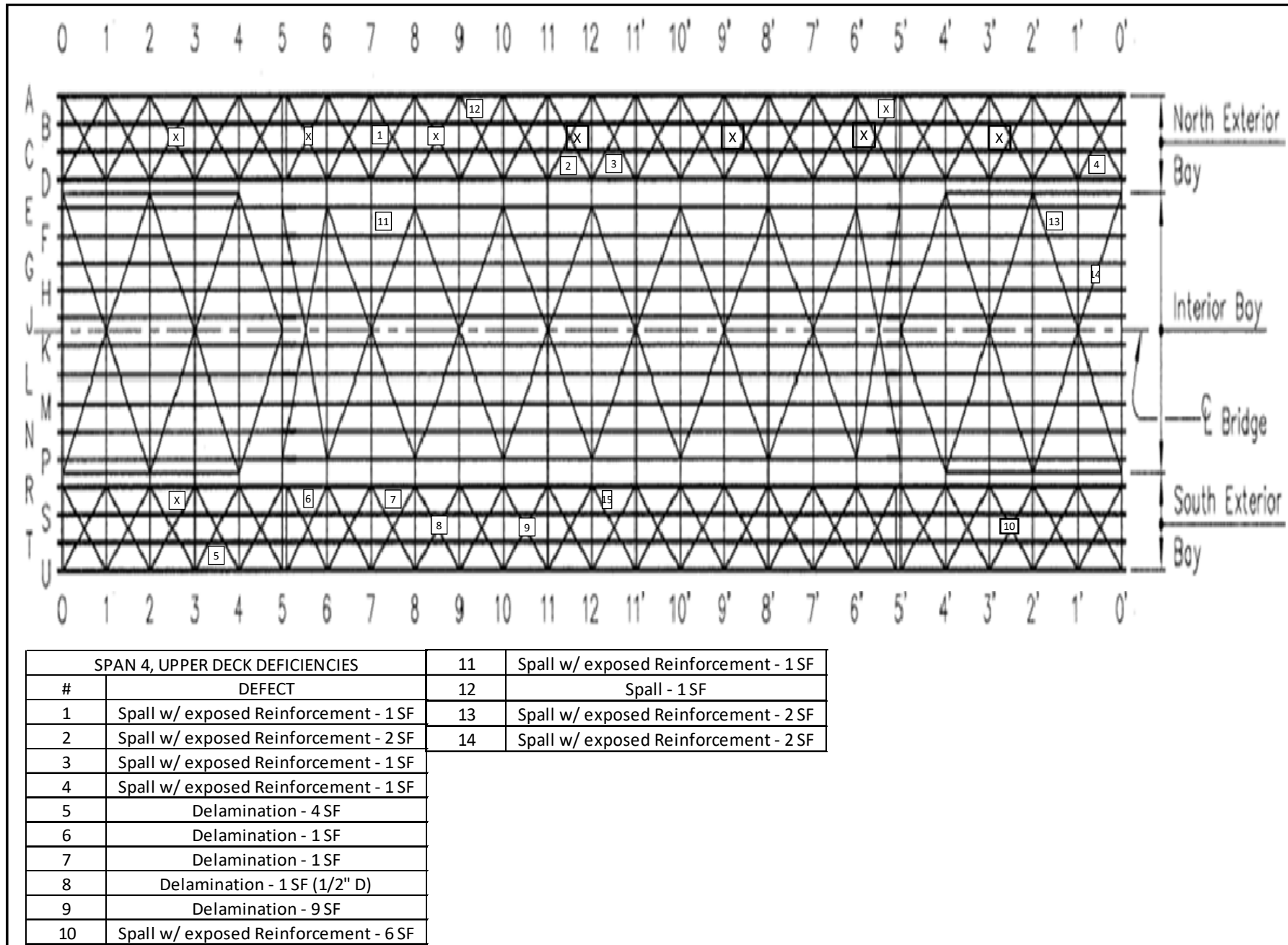
Table 7: West Station - Columns and Jack Arches				
Member Type	Column Row	Column #	Face	Defect Description
Column	A	6	NE	Northeast corner, east face delaminated for 5' height
Column	A	8	N	North face of column delaminated for full height
Column	A	11	N	Whole north face delaminated by the full height
Column	A	12	ALL	All faces delaminated or spalled, east face spalled up to 6" deep by full width with ECR
Column	A	13	NW	Northwest corner spalled 5' high x up to 12" +/- wide @ top
Column	A	17	ALL	Spalls and delamination, also incomplete repair 4' high maximum x full width
Column	A	18	NW	Northwest corner of column, full height x 6" wide delamination
Jack Arch	A	17,19	N	North face of A17-A19 jack arch spalled and delaminated with ECR
Column	A	20	N,E	4' high x full width spall in north face, east face full width x 1' high spall - incomplete
Jack Arch	A	21,22	S	Wall: west, 4' x full height spalled with ECR, lower 3' x 3' @ A22 delamination and spall
Jack Arch	A	22,23	S	Full width x full height of wall spalled with ECR
Jack Arch	A	23,24	N	Jack arch, north face, delamination for whole length
Jack Arch	B	9, 10	S	6' high x 3' +/- wide spall x 4" deep with ECR, Jack arch spalled 4' long x 1' high with ECR
Column	B	10	S	6' high x 3' +/- wide spall x 4" deep with ECR, Jack arch spalled 4' long x 1' high with ECR
Jack Arch	B	12,13	S	Underside of B12-B11 jack arch spalled/delaminated for 4' ± length. North face of arch
Jack Arch	B	13,14	S	Jack arch delaminated for whole length +/-
Jack Arch	B	14,15	S	Jack arch spalled / delaminated @ both ends
Column	B	20	ALL	Spalls @ all faces of the column
Jack Arch	B	20,21	S	Jack arch spalled for 4' near B21, south face
Column	B	24	SE	3' high x full width each face, southeast corner and south face spalled with ECR, 3" deep.
Column	B	28	SE	Southeast corner spalled 2' high x 2' wide x 2" +/- deep with ECR
Column	B	29	SW	South and west faces spalled with ECR, full width south face, 1' west face x 2'-6" +/- high
Column	C	10	N/E	8' high x 3' +/- wide spall with ECR x 4" +/- deep
Column	C	12	NE	8' high x 4" deep x full width spall
Column	C	15	ALL	Spalled up to 8" deep for most of column
Jack Arch	C	16,17	ALL	C16-C17 jack arch spalled with ECR at center of arch through the whole width of arch
Jack Arch	C	16-17	S	South face of jack arch, spalled with ECR, full length
Column	C	18	S	Southeast corner spalled, 4' high x 3' +/- wide with ECR
Column	C	23	S	4' high x full width spall with ECR 3" +/- deep
Column	C	29	NE	North end east face delaminated with spalls 5' ± high
Column	C	30	N,W,S	Typical delamination / spalls @ all 3 faces
Column	D	11	NW	Northwest corner delaminated and spalled, 6' high x 1' wide
Column	D	13	W	West face, full height spalled with ECR
Column	D	18	NE	North and east faces delaminated / spalled, 3' high x full width
Column	D	20	N	Bottom 2' spall with ECR, north face, 1' wide x 1" deep
Column	D	24	S	South face, 3' high x full width spall with ECR, 3" deep
Column	D	23	ALL	Entire bottom 3' spalled with ECR, all faces, up to 4" +/- deep
Column	D	26	ALL	Top 4' of column spalled or delaminated with ECR
Column	D	27	N,S	Top 3' delaminated on north and south faces by the full width
Column	D	30	ALL	Delaminated or spalled, top 5' +/- all faces
Jack Arch	E,F	22-27	ALL	Lines E/F from columns 22-27, there are 6 columns with 3' high x 3" deep spalls with ECR
Jack Arch	G4-H3 H4-H5	G4-G5 H4-H5	N/S	Jack arches delaminated and spalled full length of south side of G4/H5 and north side H4-H5

**APPENDIX C – OPEN SPANDREL ARCH, TRUSS,
FLOOR SYSTEM, AND SUBSTRUCTURE
DRAWINGS AND DEFICIENCIES**

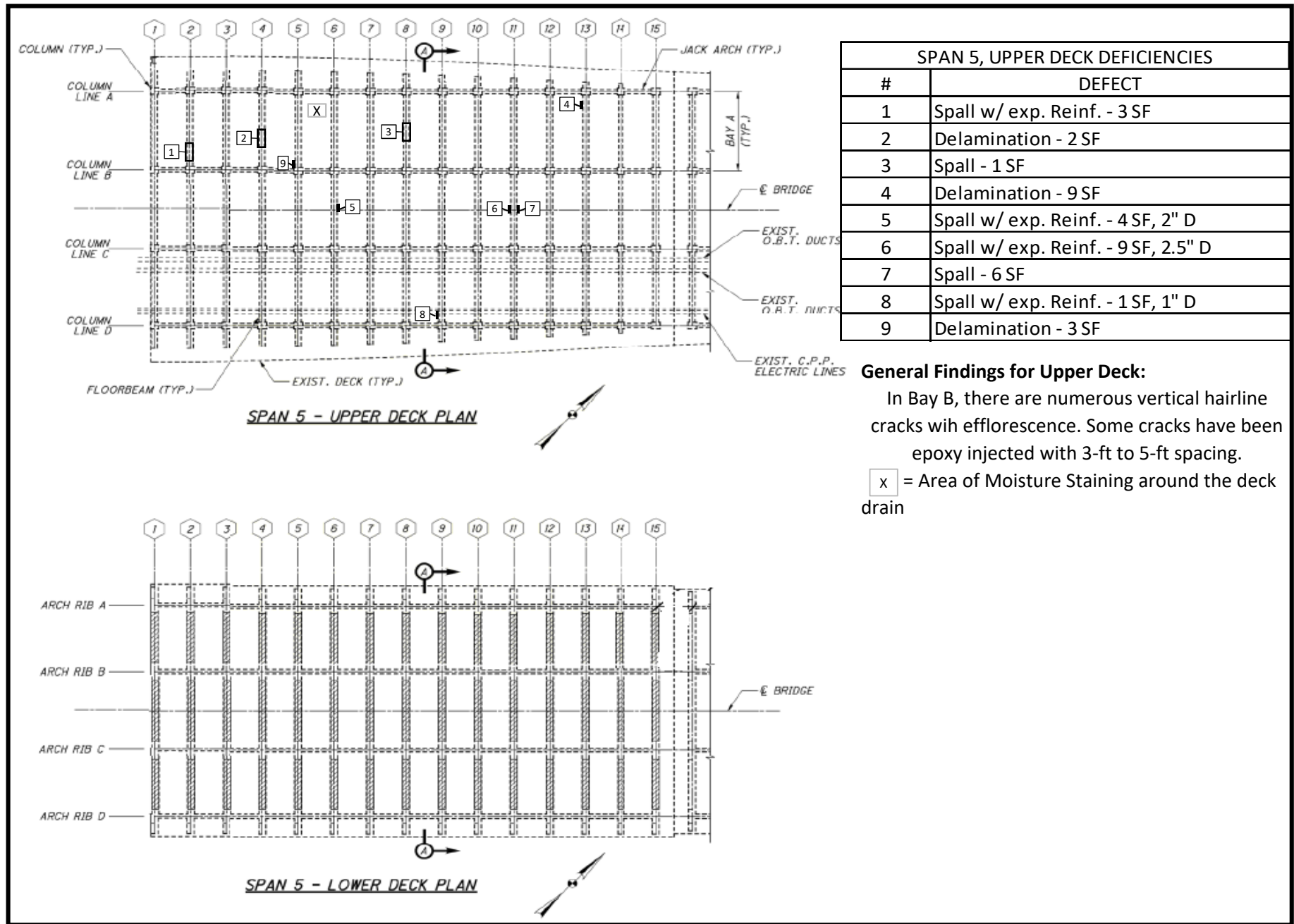








Appendix C - Open Spandrel Arch, Truss, Floor System, and Substructure Drawings and Deficiencies

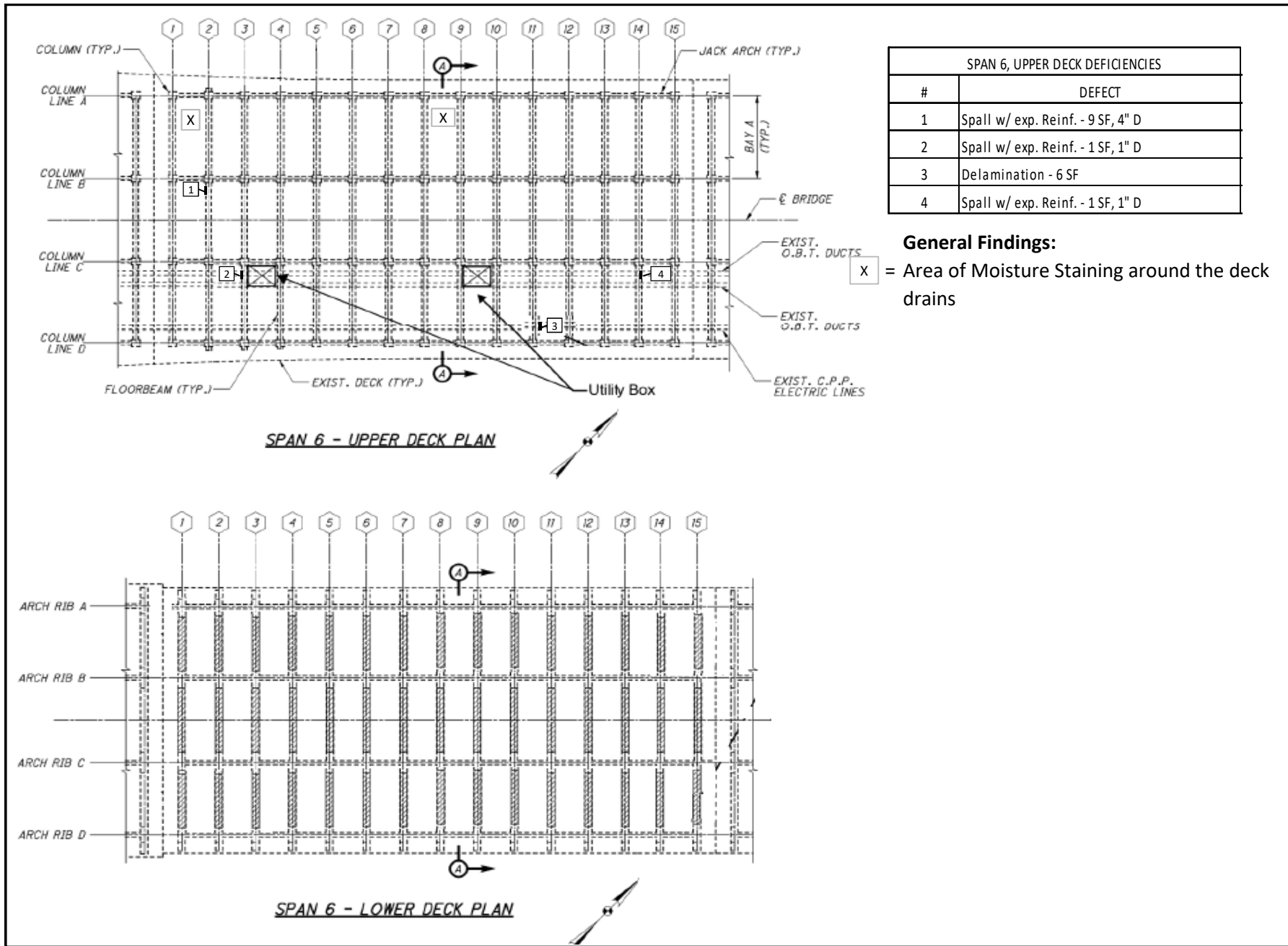


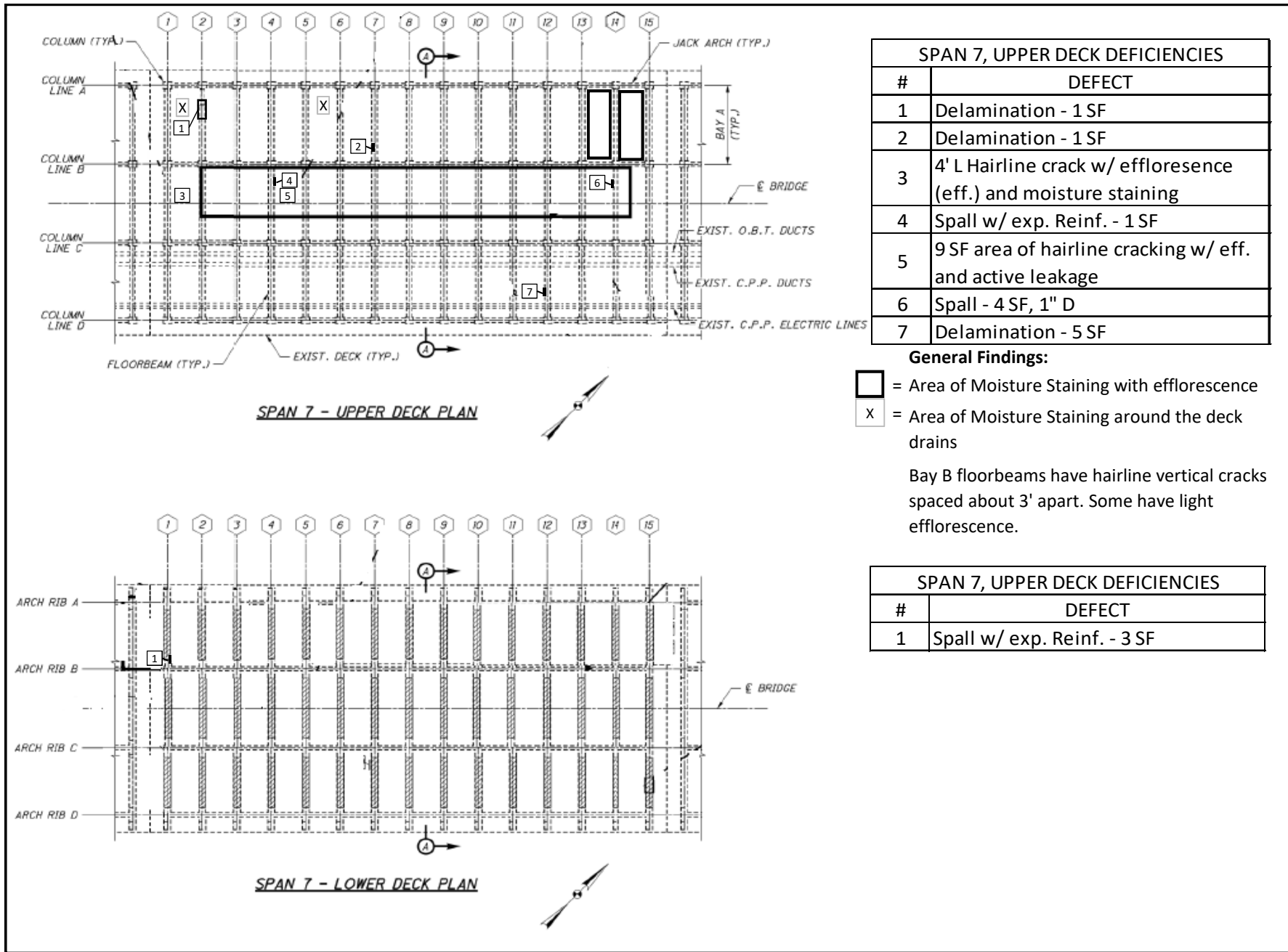
SPAN 5, UPPER DECK DEFICIENCIES	
#	DEFECT
1	Spall w/ exp. Reinf. - 3 SF
2	Delamination - 2 SF
3	Spall - 1 SF
4	Delamination - 9 SF
5	Spall w/ exp. Reinf. - 4 SF, 2" D
6	Spall w/ exp. Reinf. - 9 SF, 2.5" D
7	Spall - 6 SF
8	Spall w/ exp. Reinf. - 1 SF, 1" D
9	Delamination - 3 SF

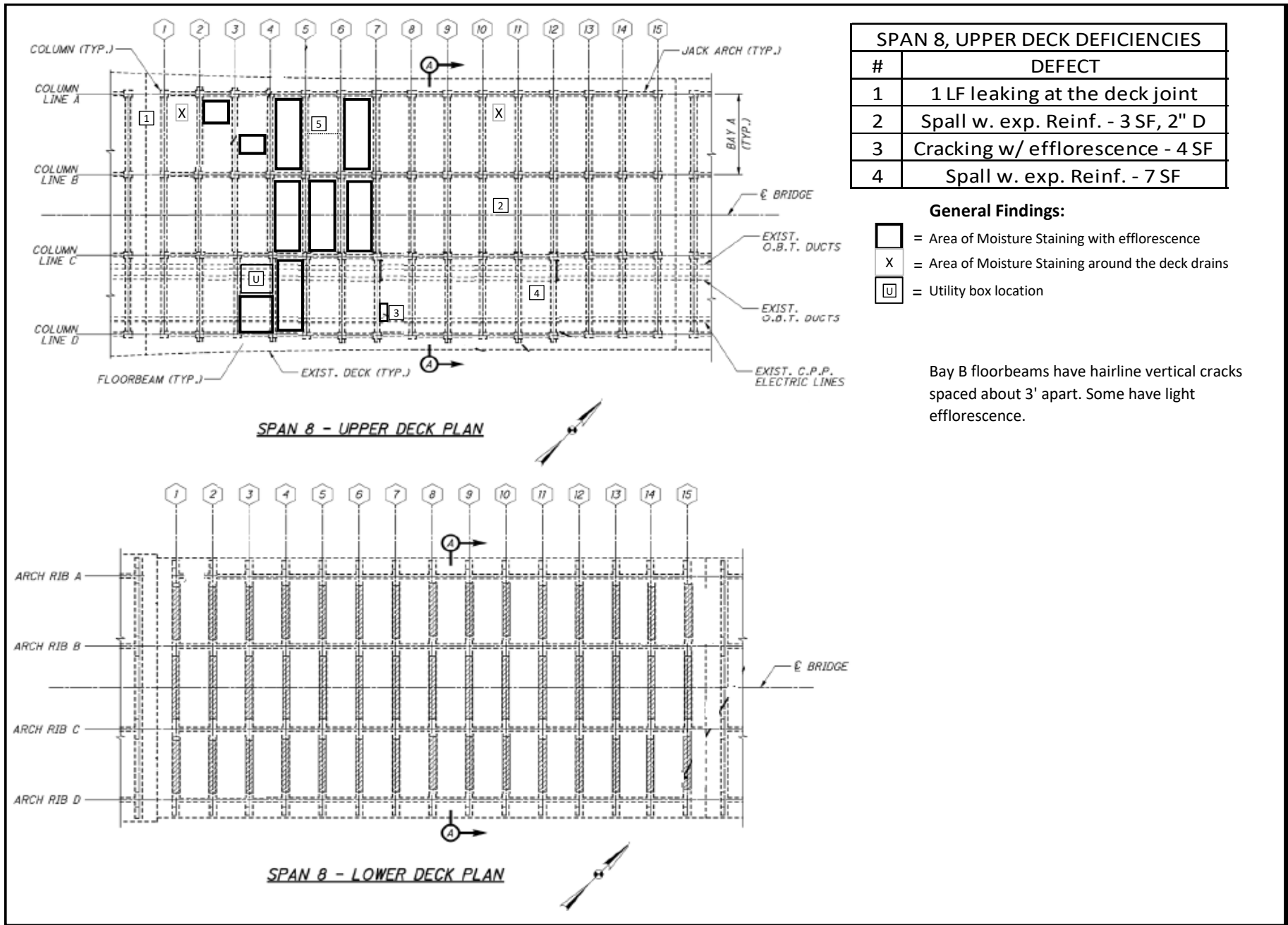
General Findings for Upper Deck:

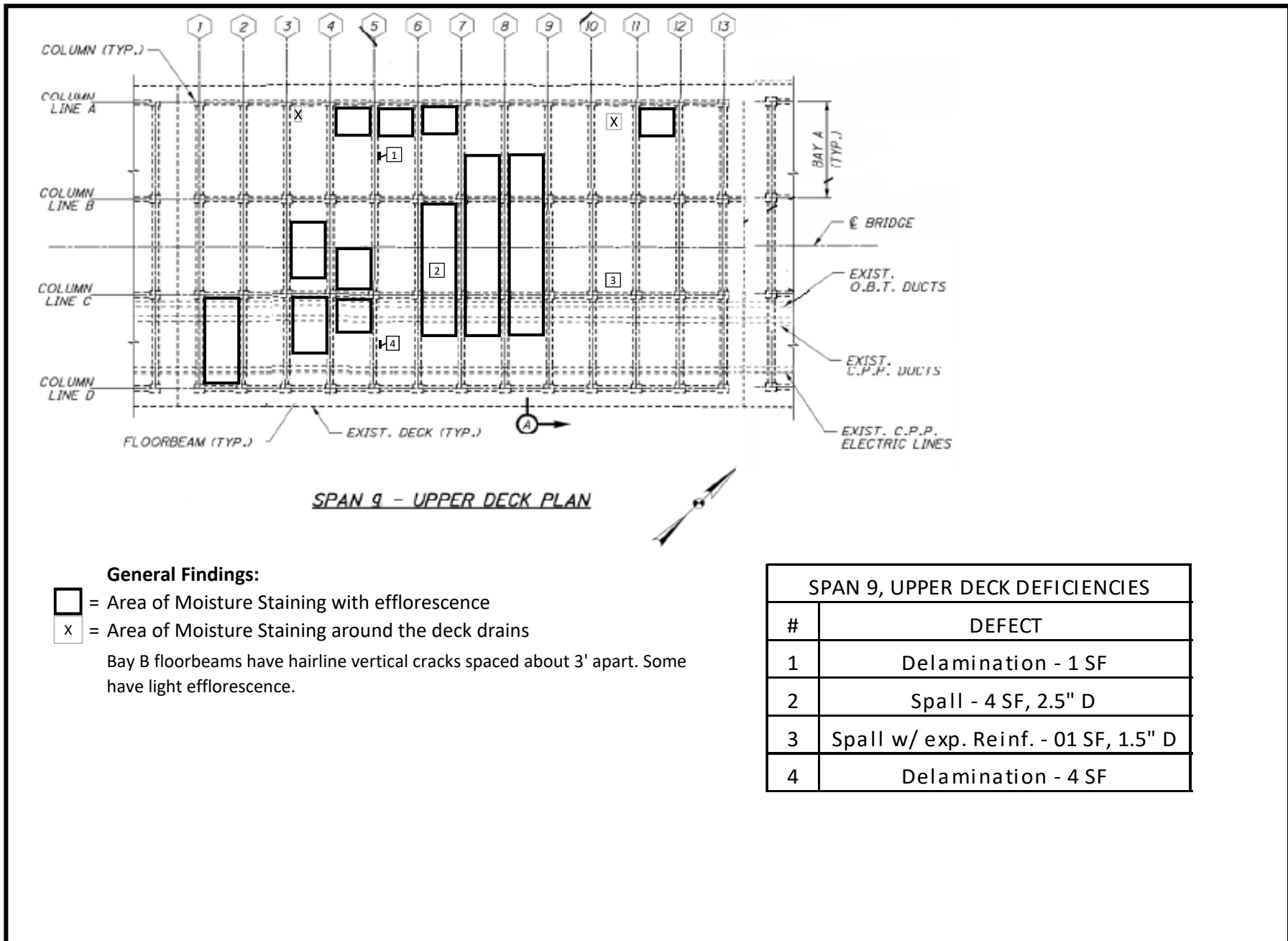
In Bay B, there are numerous vertical hairline cracks with efflorescence. Some cracks have been epoxy injected with 3-ft to 5-ft spacing.

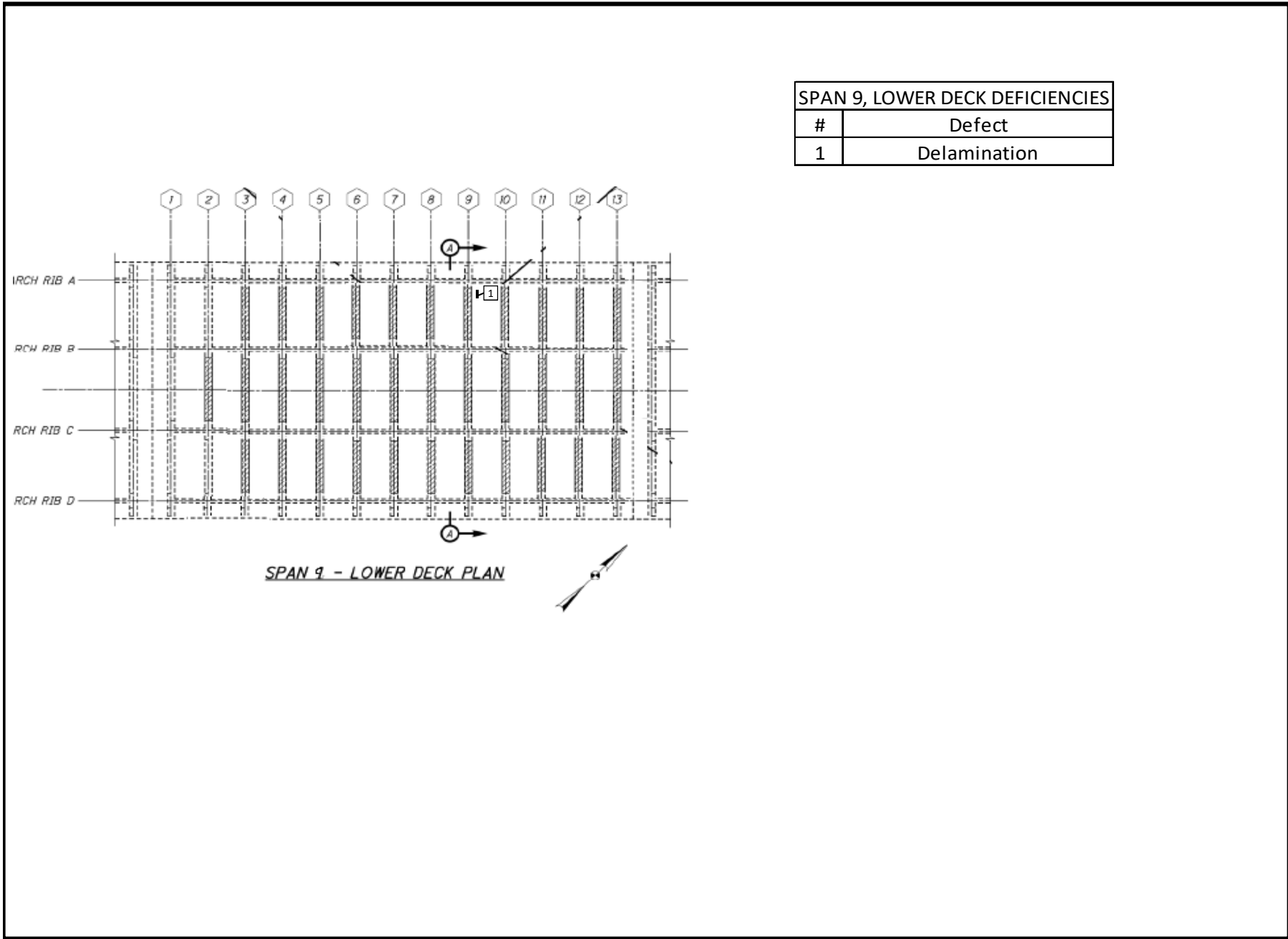
X = Area of Moisture Staining around the deck drain



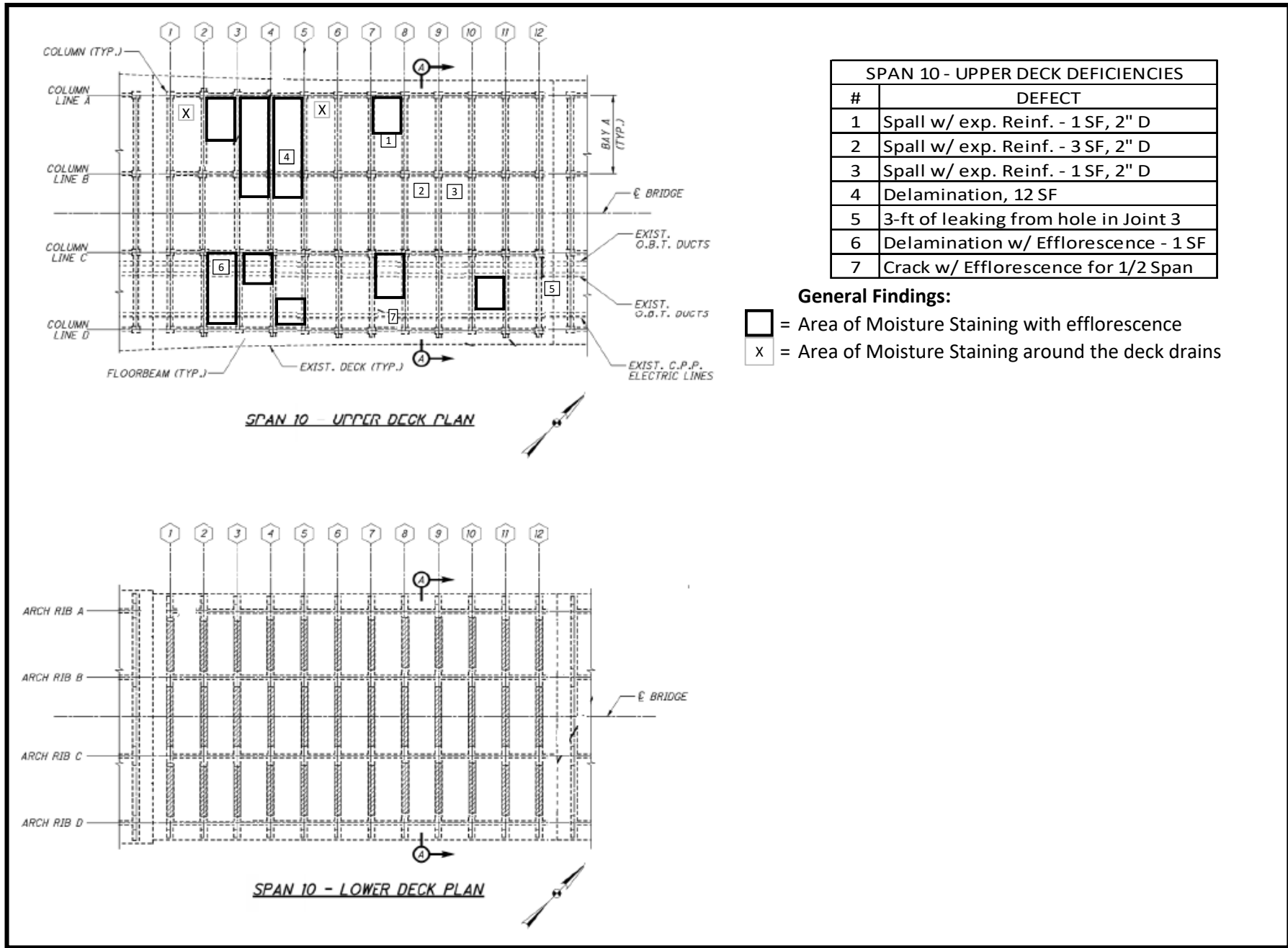


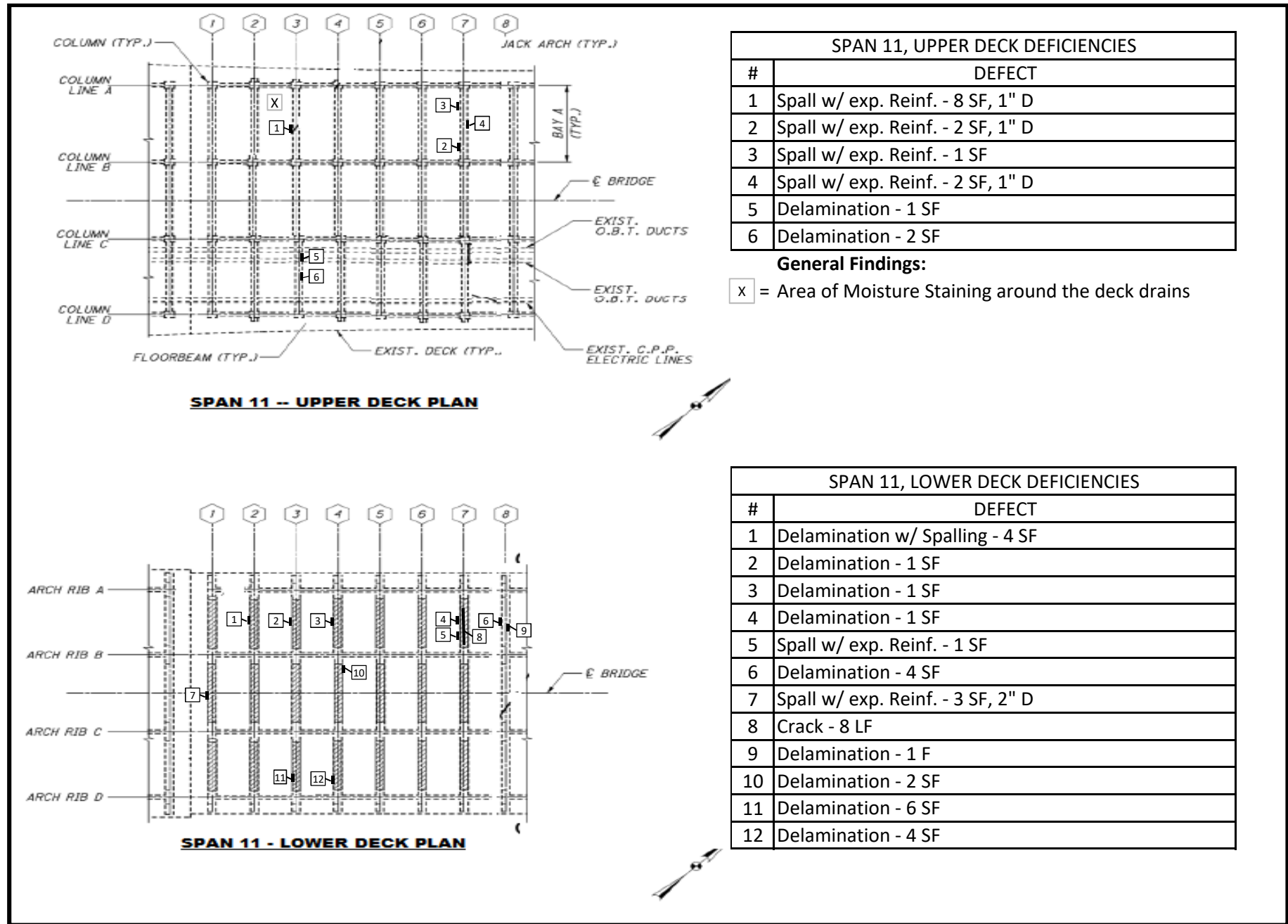


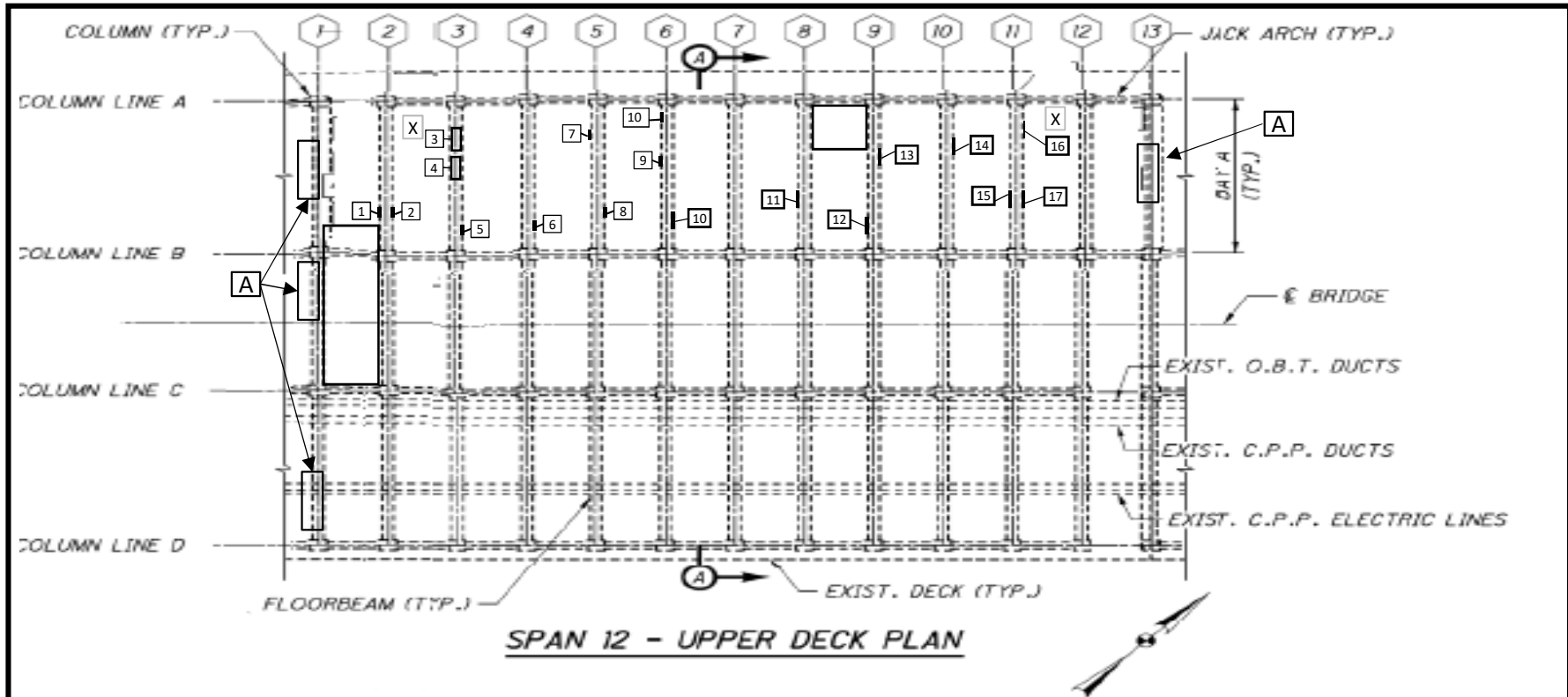




SPAN 9, LOWER DECK DEFICIENCIES	
#	Defect
1	Delamination





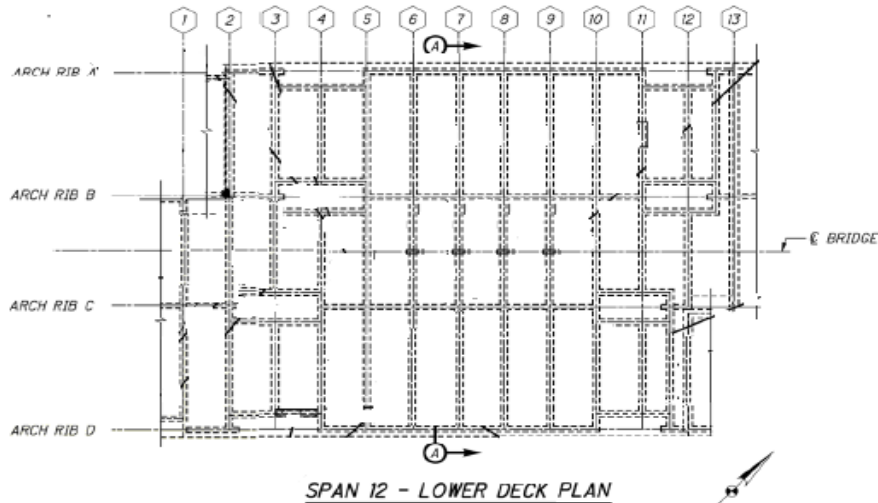


SPAN 12 - UPPER DECK PLAN

SPAN 12 - UPPER DECK DEFICIENCIES			
#	DEFECT	#	DEFECT
1	Delamination - 1 SF	10	Spall w/ exp. Reinf. - 1 SF
2	Delamination - 1 SF	11	Spall w/ exp. Reinf. - 1 SF
3	Delamination - 4 SF	12	Spall w/ exp. Reinf. - 2 SF, 1" D
4	Delamination - 3 SF	13	Spall w/ exp. Reinf. - 3 SF, 1" D
5	Delamination - 3 SF	14	Spall w/ exp. Reinf. - 13 SF
6	Spall w/ exp. Reinf. - 1 SF	15	Delamination - 6 SF
7	Spall w/ exp. Reinf. - 1 SF	16	Delamination - 1 SF
8	Delamination - 6 SF	17	Spall w/ exp. Reinf - 1 SF
9	Delamination - 27 SF		

X = Area of Moisture Staining around the deck drains
 □ = Area of Moisture Staining with efflorescence

See the next sheet for note A

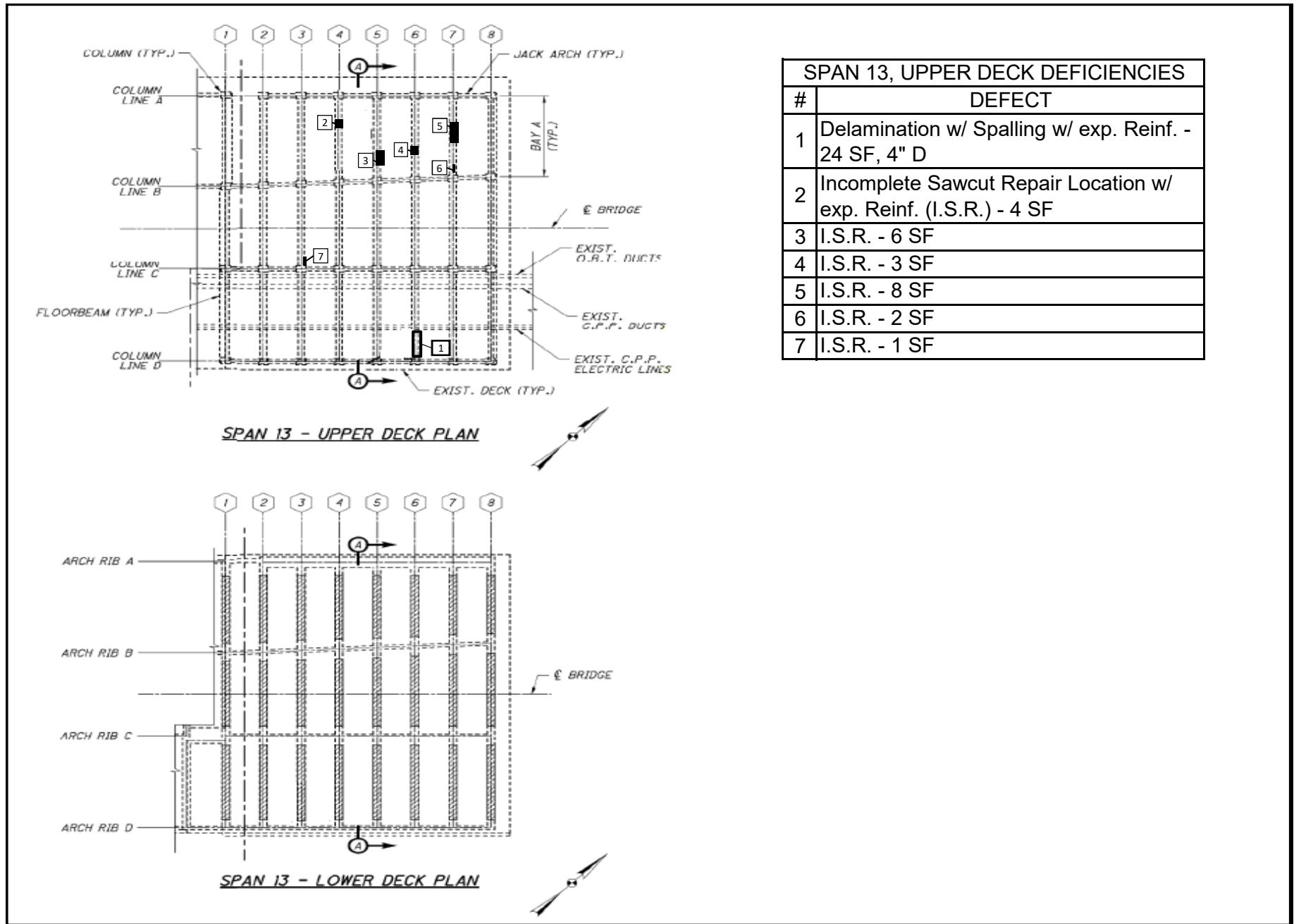


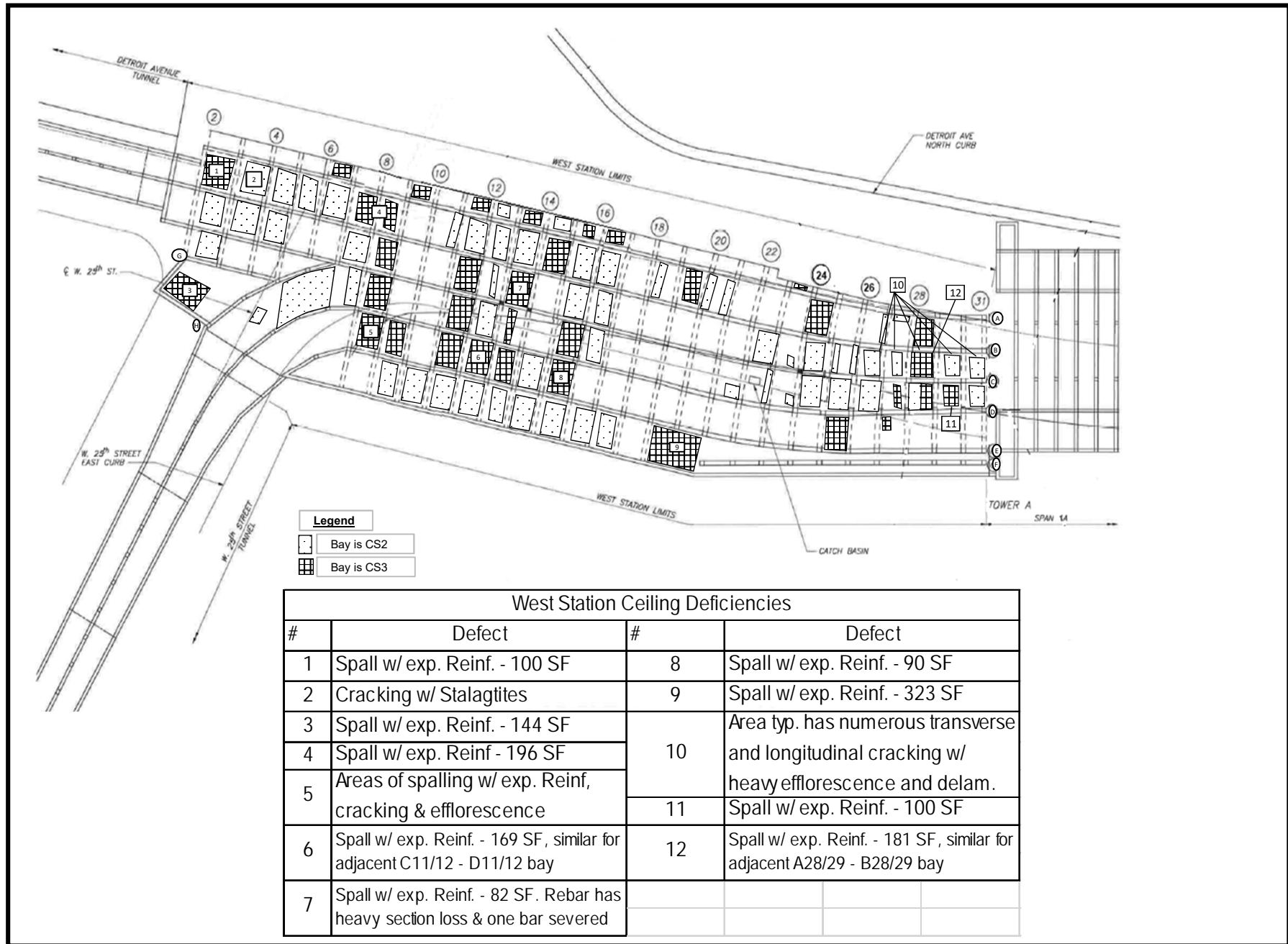
SPAN 12, LOWER DECK DEFICIENCIES				
FB #	Defect	Bay A (sf)	Bay B (sf)	Bay C (sf)
1	Delamination		0	0
	Spall w/ exp. Reinf.		0	0
2	Delamination	4	19	13
	Spall w/ exp. Reinf.	5	14	5
3	Delamination	1	3	0
	Spall w/ exp. Reinf.	0	0	0
4	Delamination	6	6	33
	Spall w/ exp. Reinf.	6	2	16
5	Delamination	8	0	17
	Spall w/ exp. Reinf.	16	0	5
6	Delamination	94	0	12
	Spall w/ exp. Reinf.	0	1	1
7	Delamination	2	0	0
	Spall w/ exp. Reinf.	0	0	0
8	Delamination	2	11	4
	Spall w/ exp. Reinf.	2	2	1
9	Delamination	22	19	45
	Spall w/ exp. Reinf.	0	0	2
10	Delamination	14	0	8
	Spall w/ exp. Reinf.	32	0	0
11	Delamination	4	0	8
	Spall w/ exp. Reinf.	7	2	0
12	Delamination	10	4	20
	Spall w/ exp. Reinf.	8	0	2
13	Delamination	0	0	
	Spall w/ exp. Reinf.	0	0	

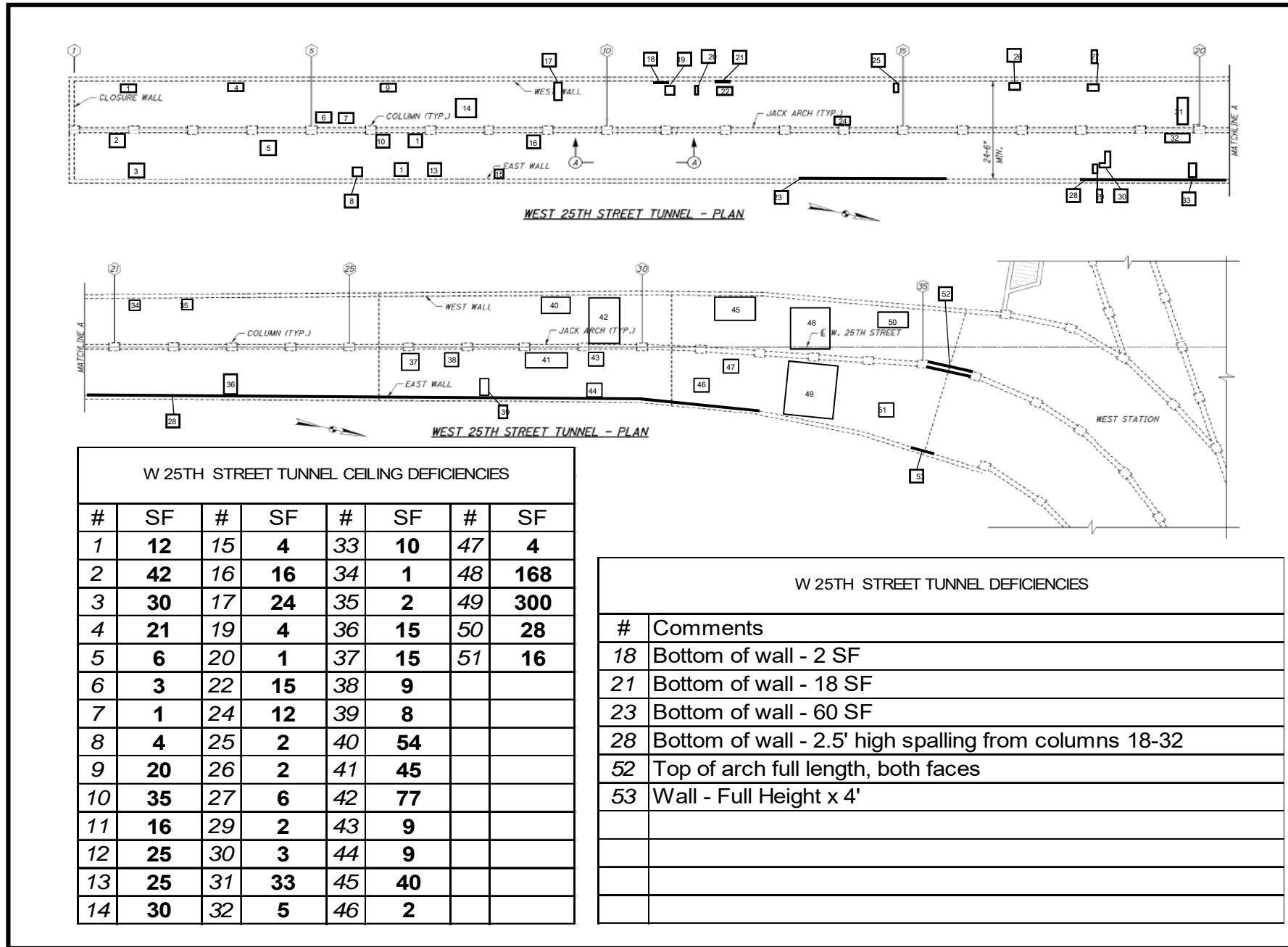
SPAN 12 - UPPER DECK FINDINGS:

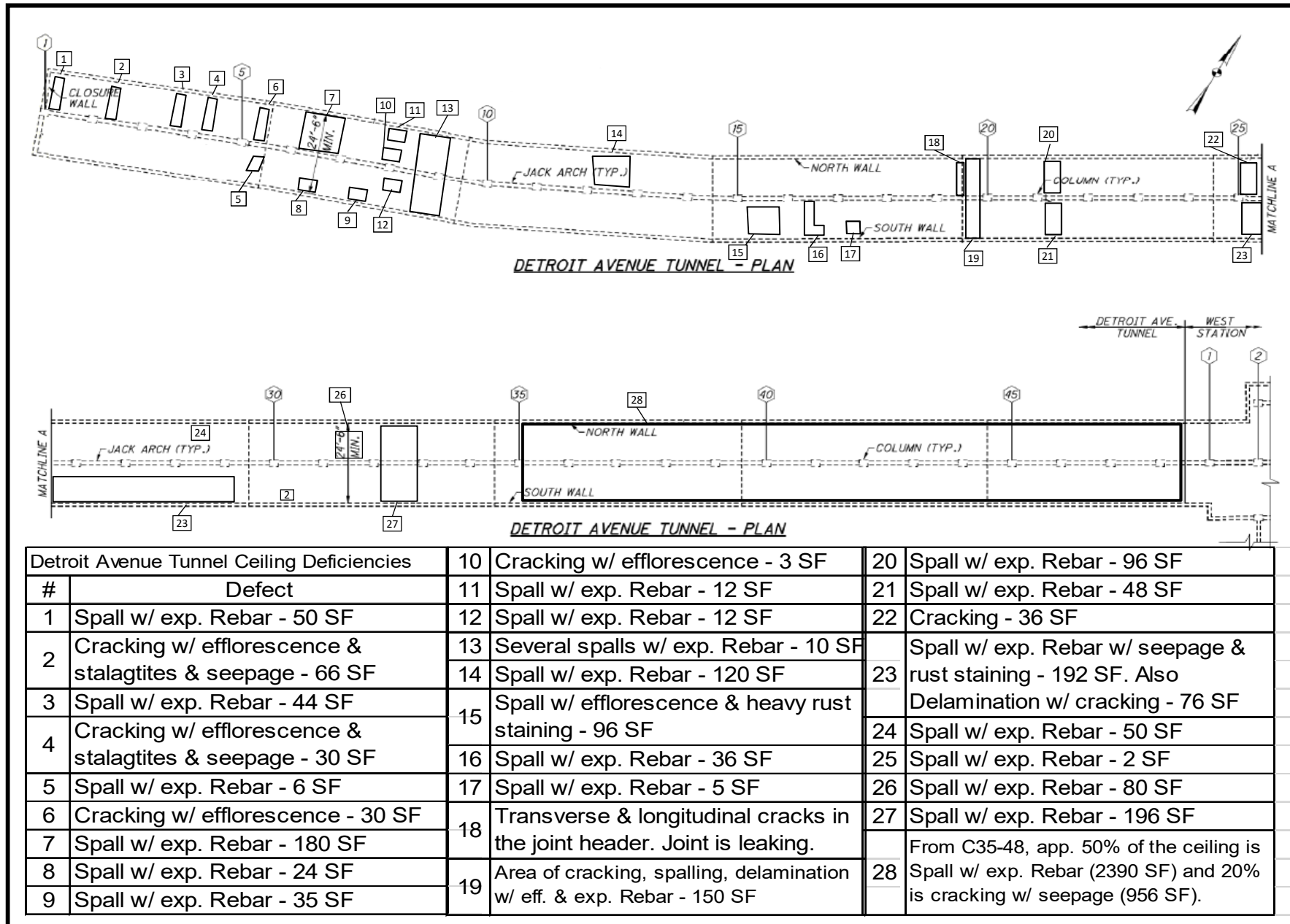
Note A: Between the deck underside and the top of the transverse floorbeam overs Piers 11 and 12, there are 3" H concrete pedestals with galvanized steel plates sitting on top and between each pedestal. In several locations these plates are missing and no longer support the deck underside, or have displaced from the bearing pedestal. See Table below for locations and description of displaced plates.

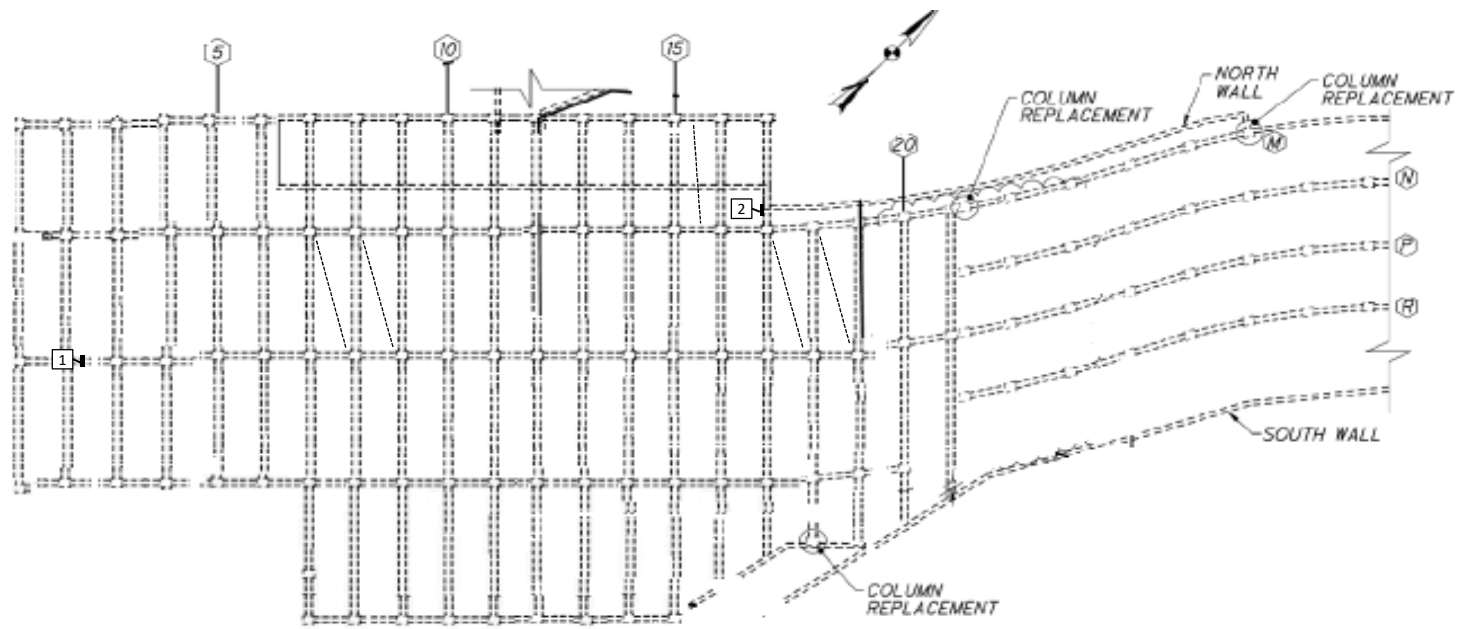
Joint	Location	Description
Pier 11	Ribs A-B	One plate missing and one displaced 4" east
	Ribs B-C	Two plates missing and two plate displaced up to 2" west
	Ribs C-D	Six plates displaced up to 6.5" west
Pier 12	Ribs A-B	One plate missing and two displaced up to 1" east





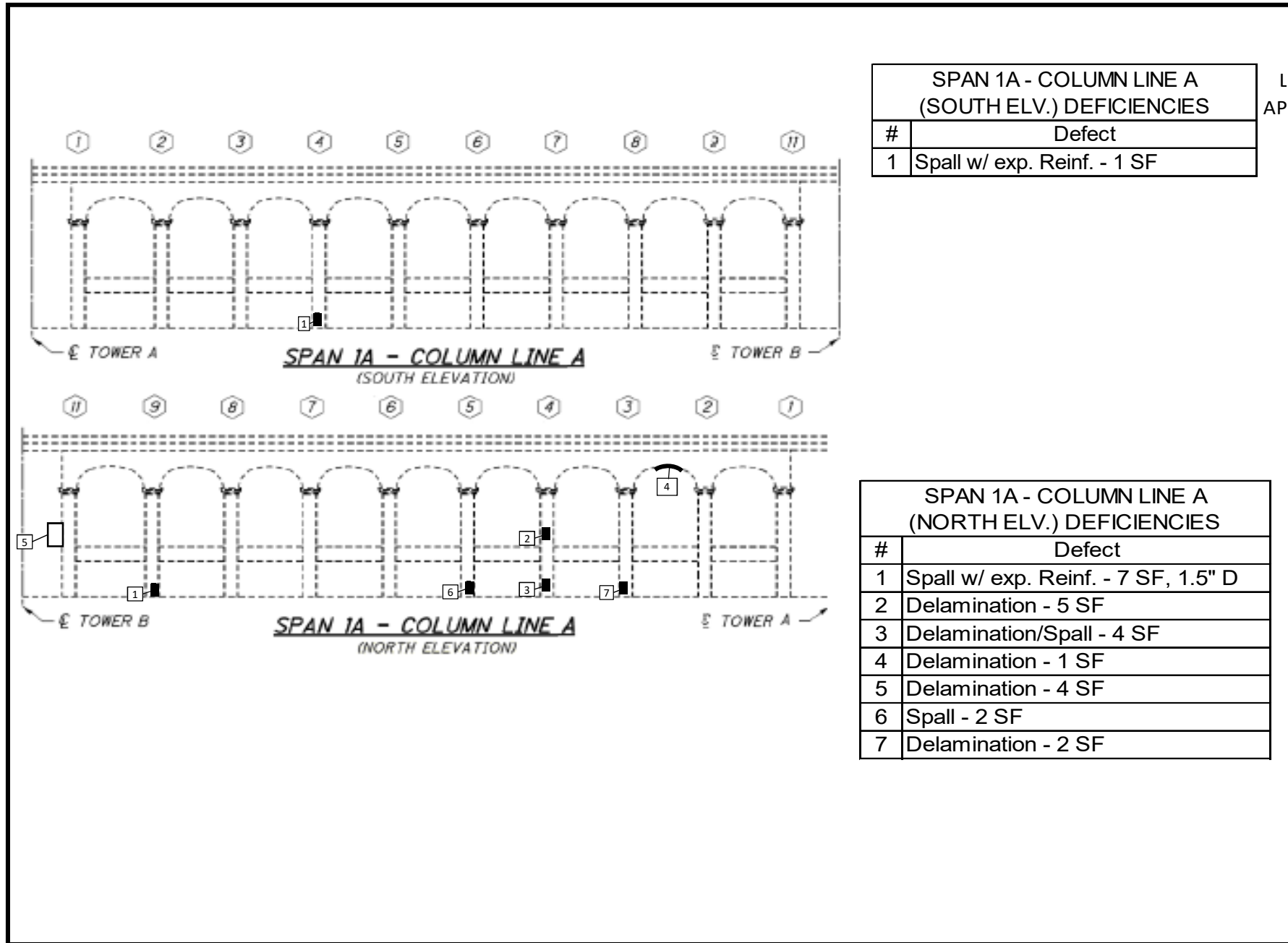


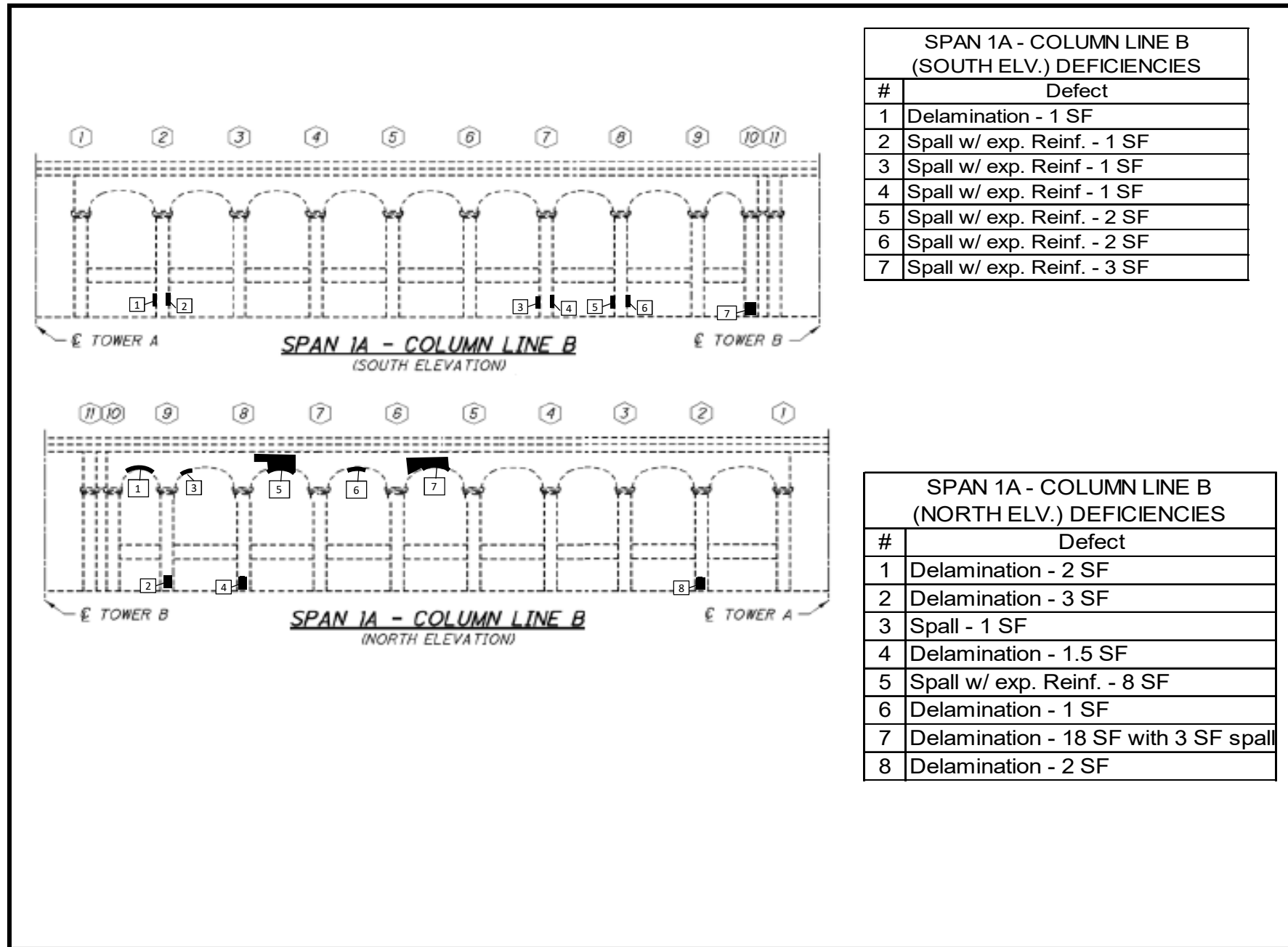


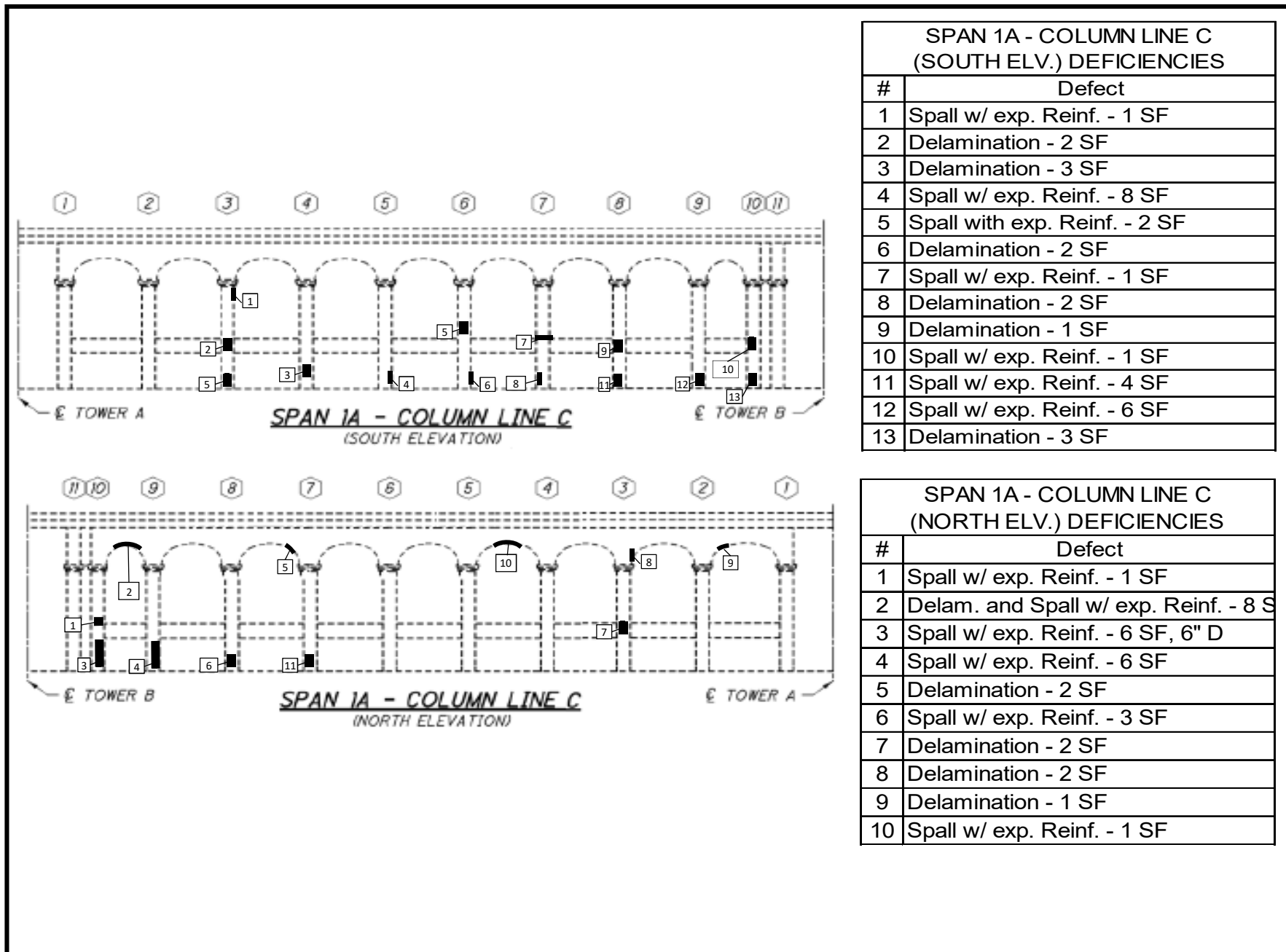


EAST STATION & SUBWAY - PLAN

EAST STATION & SUBWAY DEFICIENCIES	
#	DEFECT
1	Spall w/ exp. Reinf. - 1 SF, 1" D (underside of arch)
2	Spall - 24 SF (in wall) and Spall - 6 SF (in btm of FB)

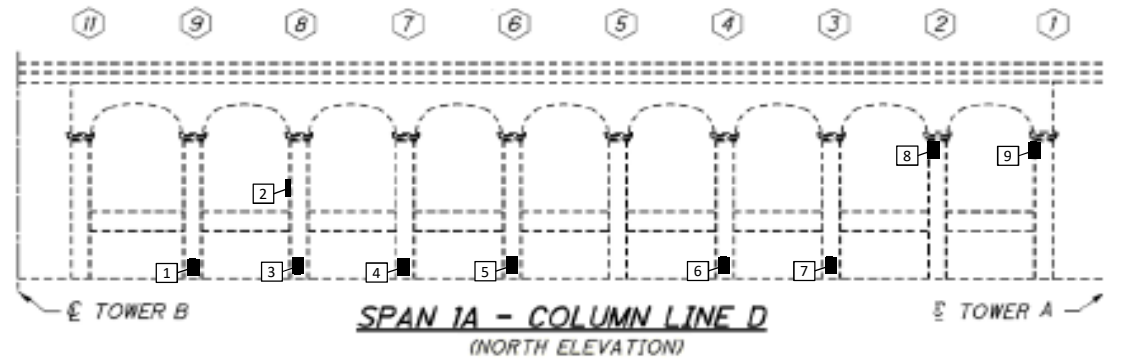
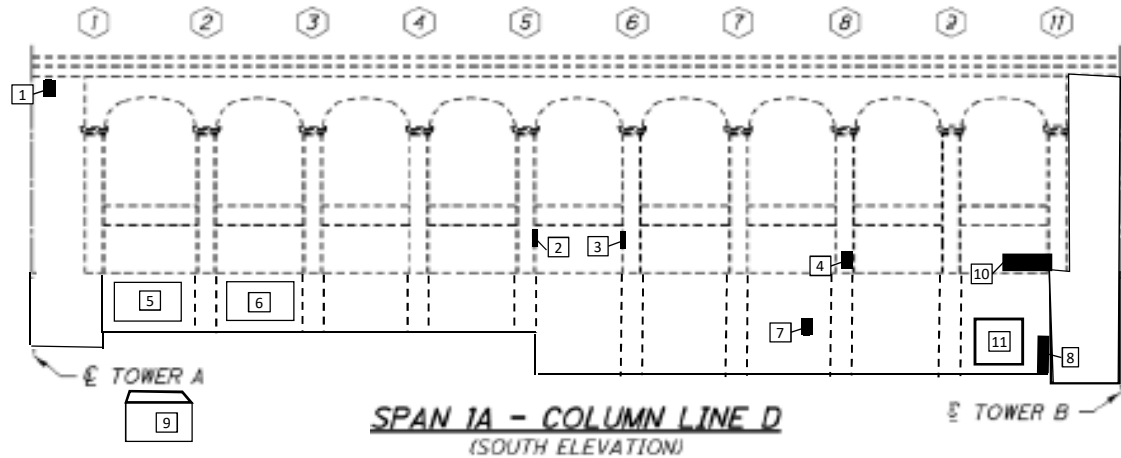


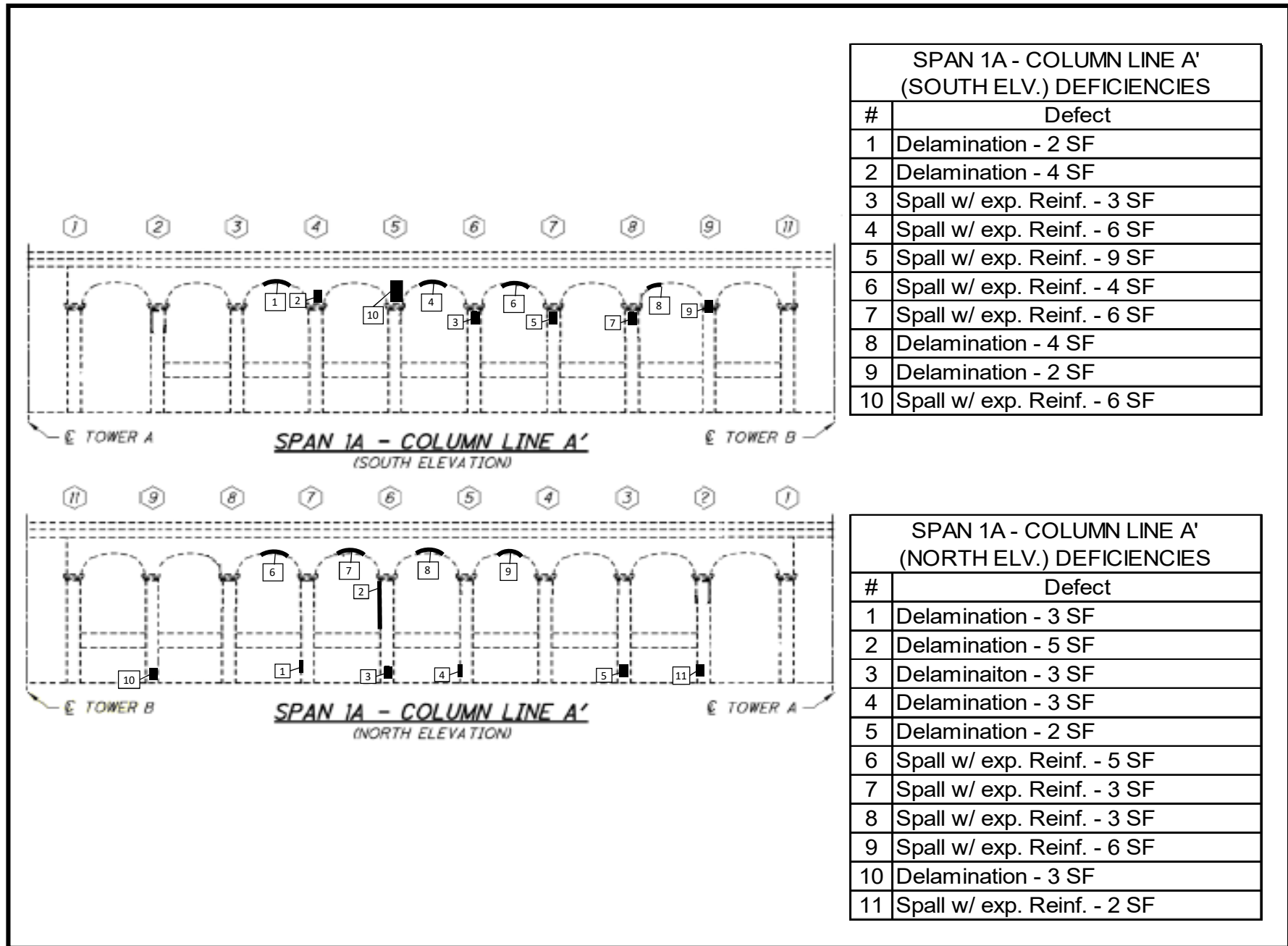




SPAN 1A - COLUMN LINE D (SOUTH ELV.) DEFICIENCIES	
#	Defect
1	Face of cap has a 1'-4" W x Full H x 6" D spall
2	Delamination - 6 SF
3	Spall w/ exp. Reinf. - 3 SF
4	Delamination - 4 SF
5	Spall w/ exp. Reinf. - 16 SF, within a Delamination - 42 SF
6	Delamination - 24 SF
7	Delamination - 6 SF
8	Spall - 5 SF, 8" D
9	Embankment along the south side of Span 1A has significant erosion for the full length. At the west end, there is a 15' Diameter x 4' D erosion ditch around the manhole. An erosion ditch extends from the manhole towards the east typ. 3' W x 2' D.

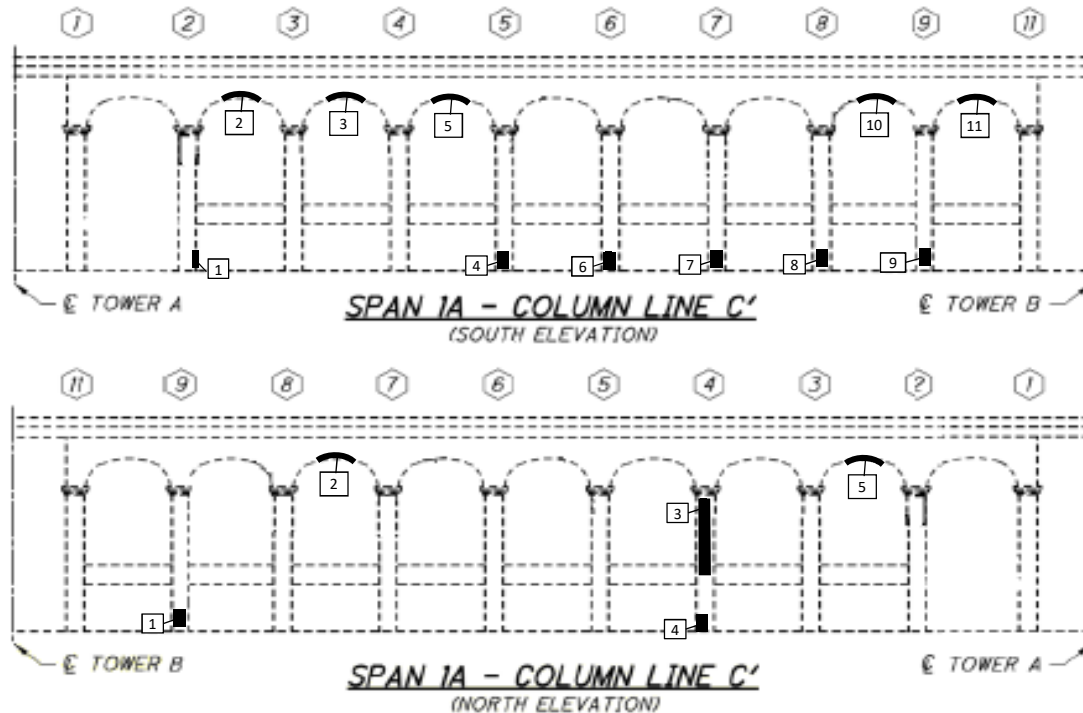
SPAN 1A - COLUMN LINE D (NORTH ELV.) DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf - 7 SF
2	Delamination - 1 SF
3	Spall w/ exp. Reinf - 6 SF
4	Spall w/ exp. Reinf - 6 SF
5	Spall w/ exp. Reinf - 4 SF
6	Spall w/ exp. Reinf. - 8 SF
7	Delamination - 6 SF
8	Delamination - 6 SF
9	Delamination - 6 SF





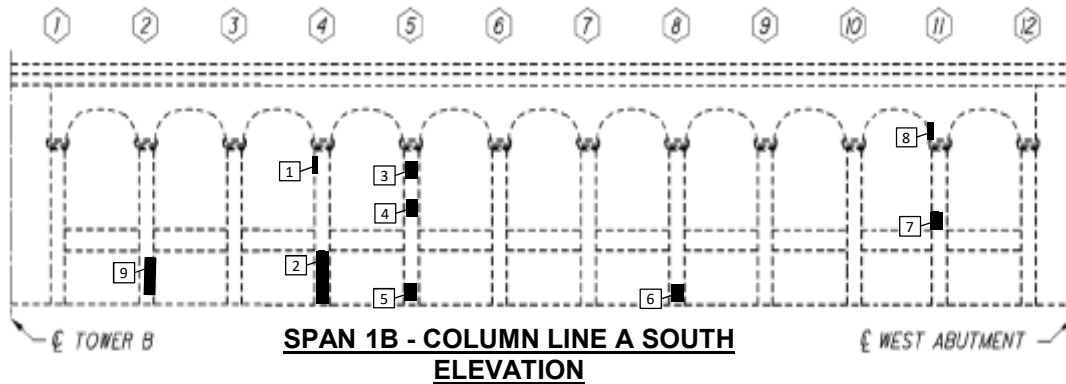
SPAN 1A - COLUMN LINE C' (SOUTH ELV.) DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 2 SF
2	Spall w/ exp. Reinf. - 6 SF
3	Spall w/ exp. Reinf. - 2 SF
4	Spall - 6.5 SF, 3" D
5	Spall w/ exp. Reinf. - 4 SF
6	Spall w/ exp. Reinf. - 3 SF
7	Spall w/ exp. Reinf. - 4 SF
8	Delamination - 4 SF
9	Delamination - 4 SF
10	Spall w/ exp. Reinf. - 3 SF
11	Spall w/ exp. Reinf. - 3 SF

SPAN 1A - COLUMN LINE C' (NORTH ELV.) DEFICIENCIES	
#	Defect
1	Delamination - 4 SF
2	Delamination - 1 SF
3	Spalling w/ exp. Reinf. And Delamination - 17 SF
4	Delamination - 2 SF
5	Delamination - 2 SF



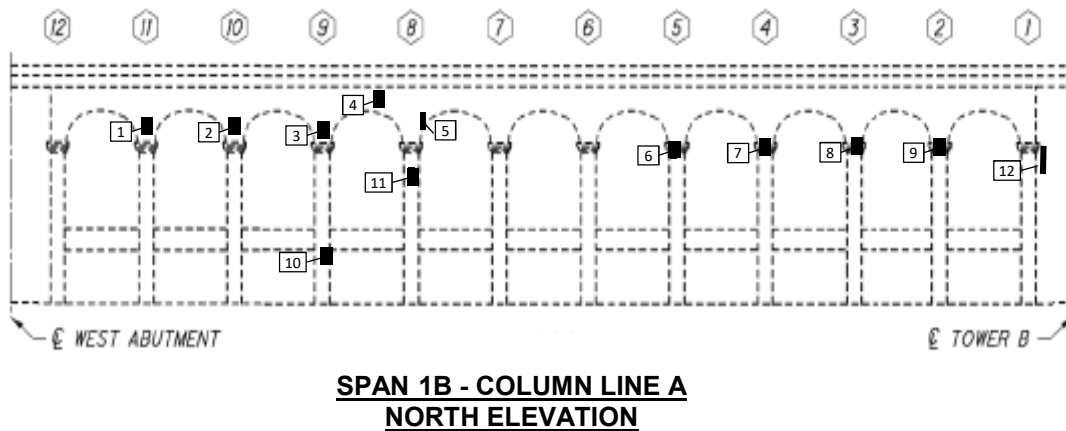
**SPAN 1B - COLUMN LINE A
(SOUTH ELV.) DEFICIENCIES**

#	Defect
1	Delamination - 2 SF
2	Spall w/ exp. Reinf. - 8 SF
3	Delamination - 4 SF
4	Delamination - 2 SF
5	Spall w/ exp. Reinf. - 8 SF
6	Delamination - 5 SF
7	Spall w/ exp. Reinf. - 4 SF
8	Delamination - 2 SF
9	Delamination - 4 SF



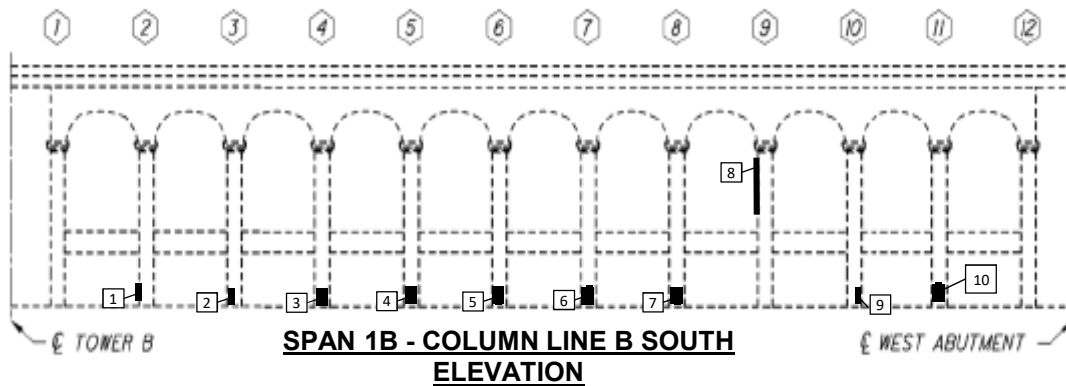
**SPAN 1B - COLUMN LINE A
(NORTH ELV.) DEFICIENCIES**

#	Defect
1	Delamination - 2 SF
2	Spall w/ exp. Reinf. - 1 SF (on bracket)
3	Spall w/ exp. Reinf. - 2 SF (on bracket)
4	Delamination - 1 SF
5	Spall w/ exp. Reinf. - 2 SF
6	Delamination - 2 SF
7	Spall w/ exp. Reinf. - 2 SF
8	Delam. - 2 SF (underside of bracket)
9	Spall w/ exp. Reinf. - 1 SF, 3" D
10	Spall w/ exp. Reinf. - 1 SF
11	Delamination - 1 SF
12	Spall w/exp. Reinf. 3 SF, 4" D



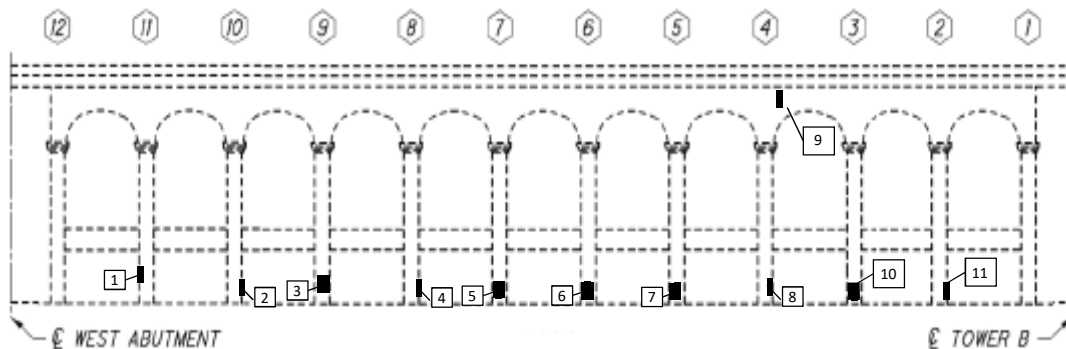
SPAN 1B - COLUMN LINE B
(SOUTH ELV.) DEFICIENCIES

#	Defect
1	Spall w/ exp. Reinf - 1 SF
2	Spall w/ exp. Reinf. - 4 SF
3	Delamination - 3 SF
4	Delamination - 4 SF
5	Spall w/ exp. Reinf. - 5 SF
6	Spall w/ exp. Reinf. - 3 SF
7	Delamination - 6 SF
8	Delamination - 6 SF
9	Spall w/ exp. Reinf. - 2 SF
10	Delamination - 4 SF



SPAN 1B - COLUMN LINE B
(NORTH ELV.) DEFICIENCIES

#	Defect
1	Spall w/ exp. Reinf. - 4 SF
2	Spall w/ exp. Reinf. - 4 SF
3	Spall w/ exp. Reinf. - 5 SF
4	Delamination - 6 SF
5	Spall w/ exp. Reinf. - 4 SF
6	Delamination - 4 SF
7	Spall w/ exp. Reinf. - 5 SF
8	Spall w/ exp. Reinf. - 4 SF
9	Delamination - 3 SF
10	Spall w/ exp. Reinf. - 8 SF
11	Spall w/ exp. Reinf. - 2 SF

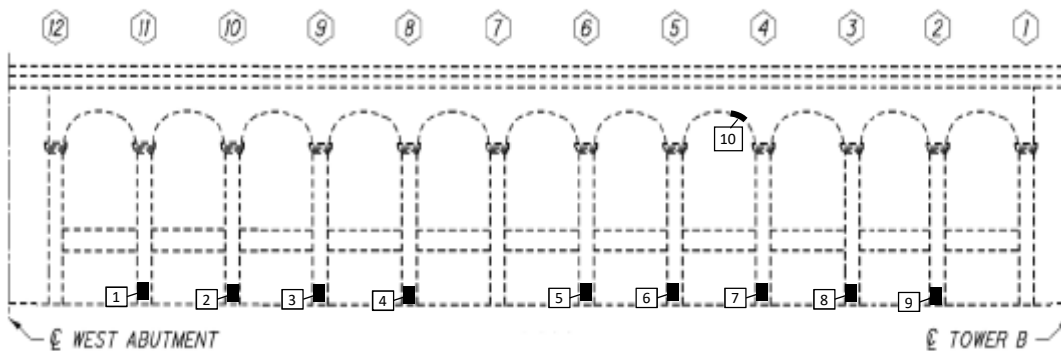
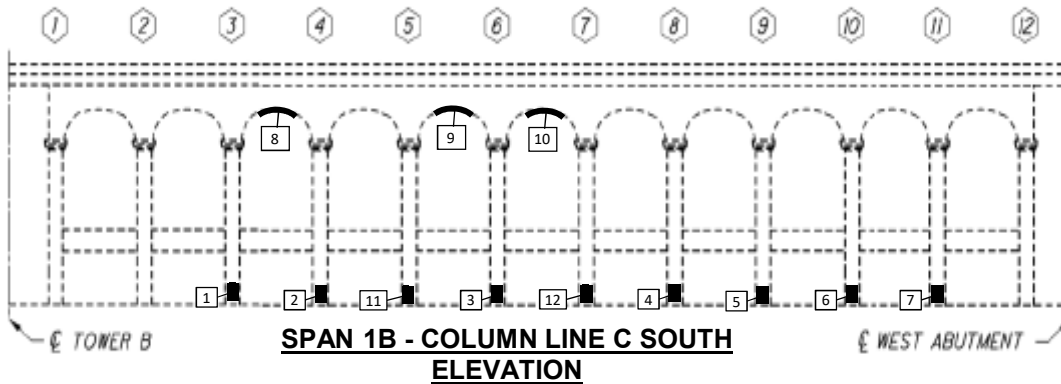


**SPAN 1B - COLUMN LINE C
(SOUTH ELV.) DEFICIENCIES**

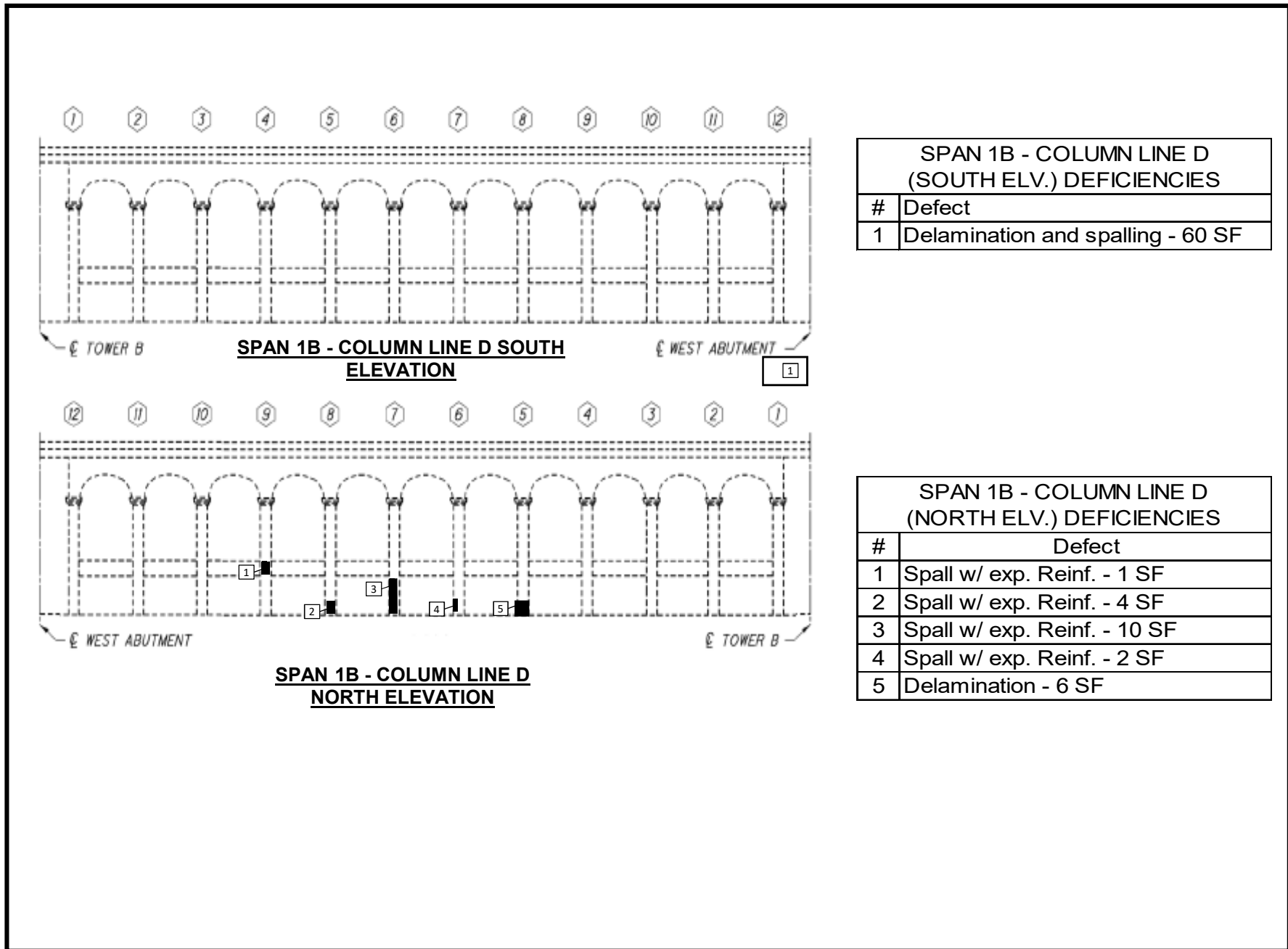
#	Defect
1	Spall w/ exp. Reinf. - 4 SF
2	Spall w/ exp. Reinf. - 5 SF
3	Spall w/ exp. Reinf. - 6 SF
4	Delamination - 4 SF
5	Spall w/ exp. Reinf. - 6 SF
6	Delamination - 6 SF
7	Spall w/ exp. Reinf. - 3 SF
8	Delamination - 2 SF
9	Spall w/ exp. Reinf. - 2 SF
10	Spall w/ exp. Reinf. - 4 SF
11	Delamination - 2 SF
12	Delamination - 15 SF

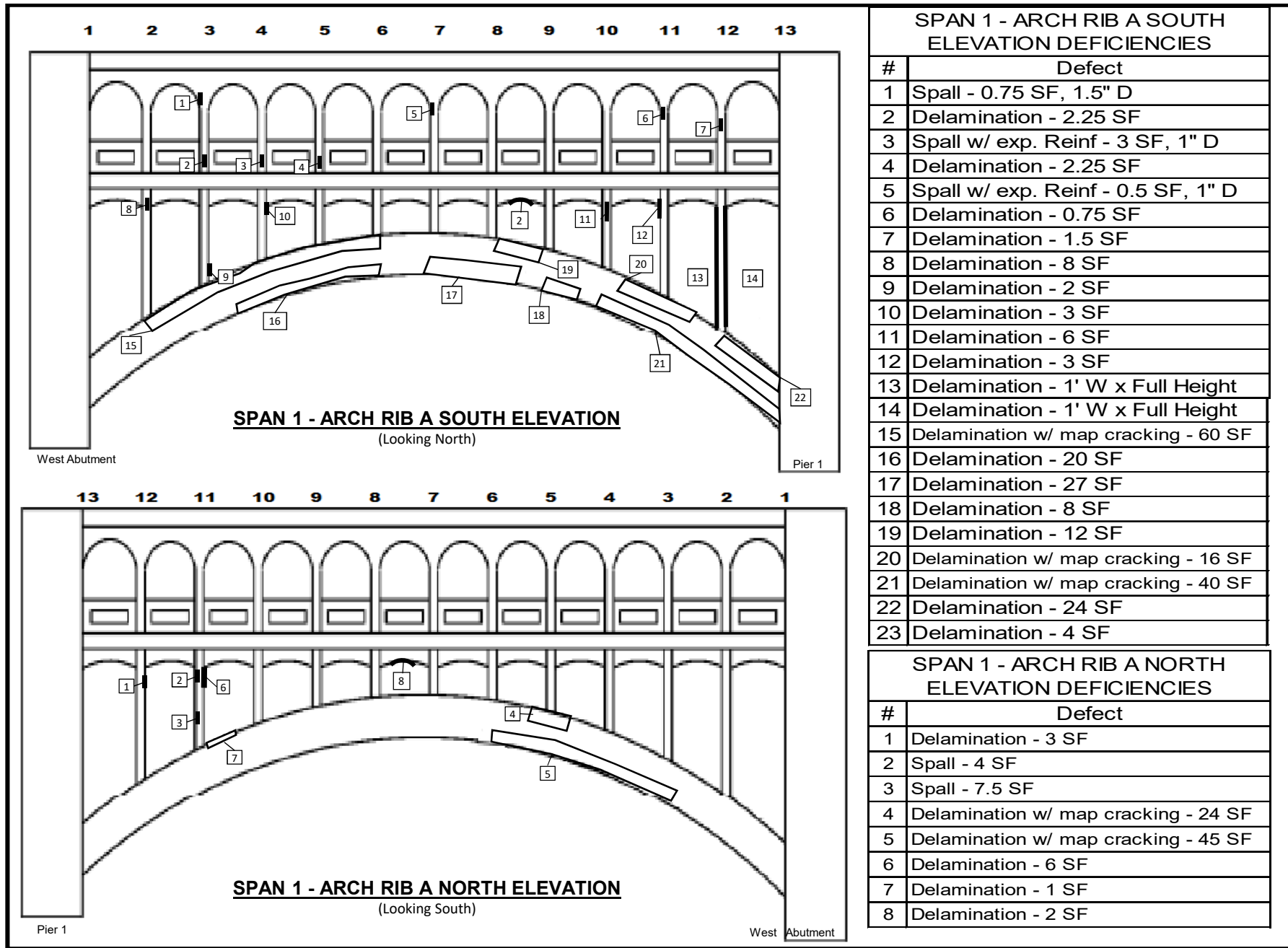
**SPAN 1B - COLUMN LINE C
(NORTH ELV.) DEFICIENCIES**

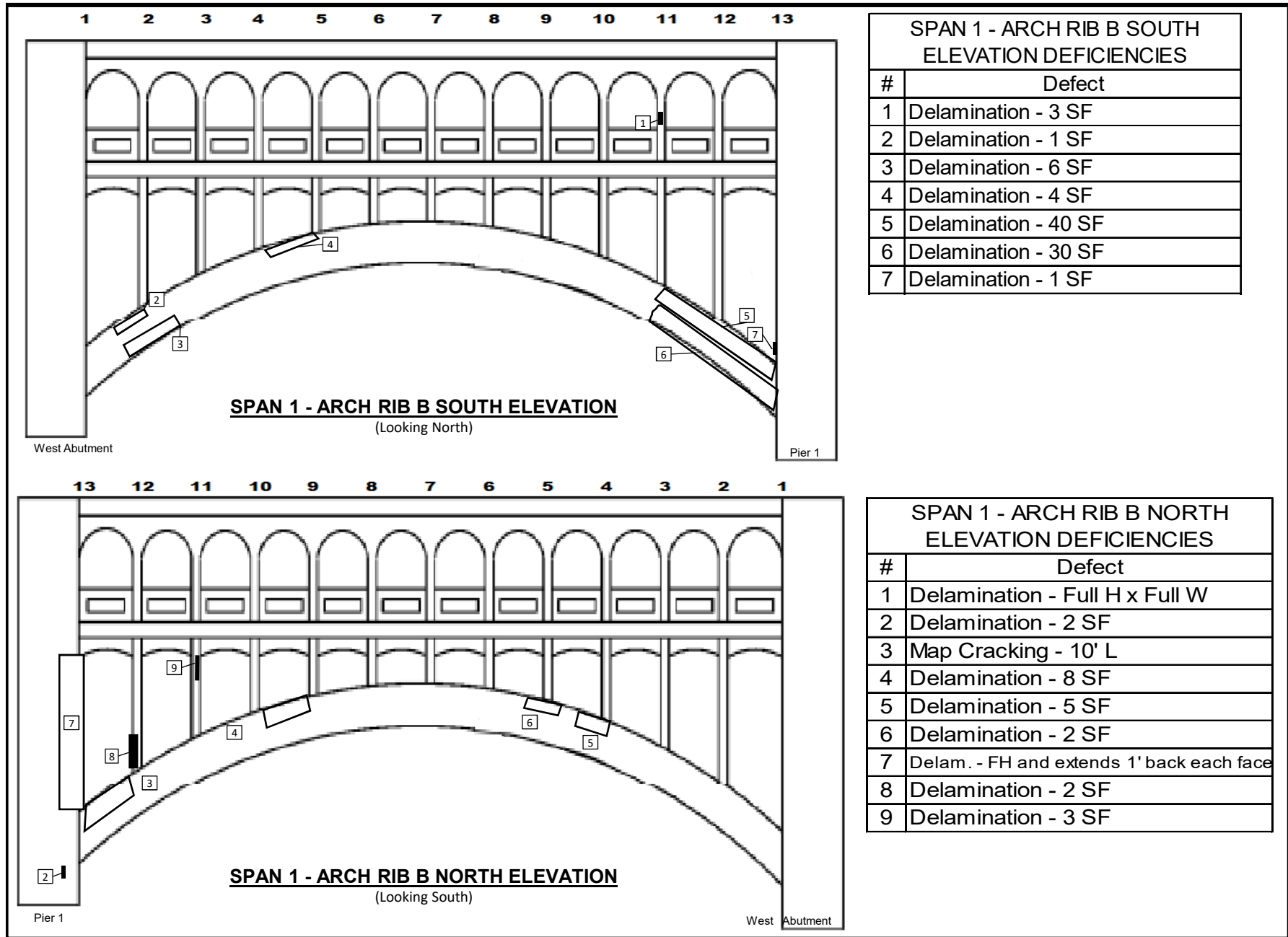
#	Defect
1	Delamination - 4 SF
2	Delamination - 4 SF
3	Delamination - 4 SF
4	Delamination - 4 SF
5	Spall w/ exp. Reinf. - 4 SF
6	Delamination - 6 SF
7	Delamination - 2 SF
8	Spall w/ exp. Reinf. - 4 SF
9	Delamination - 4 SF
10	Spall - 2 SF

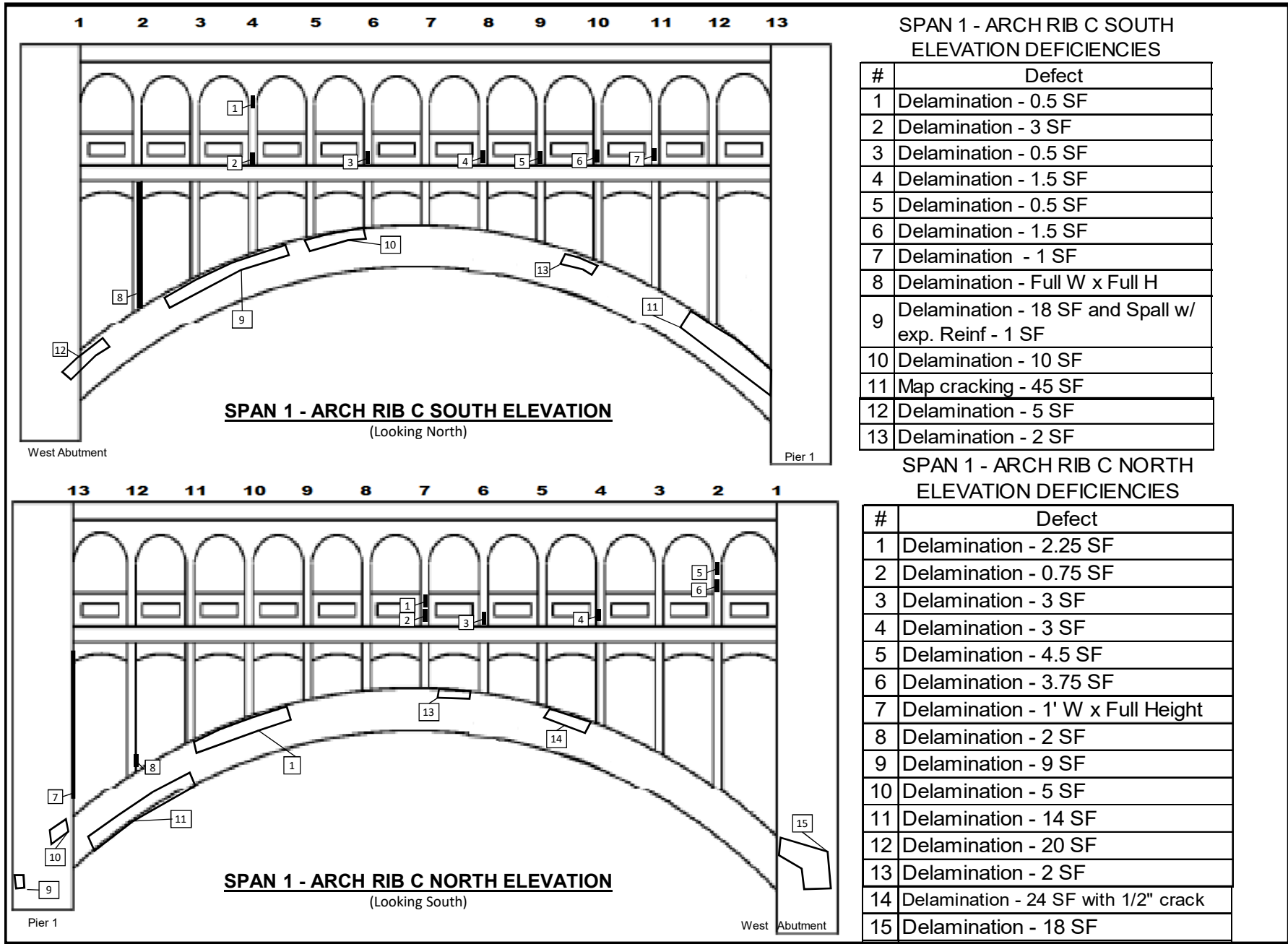


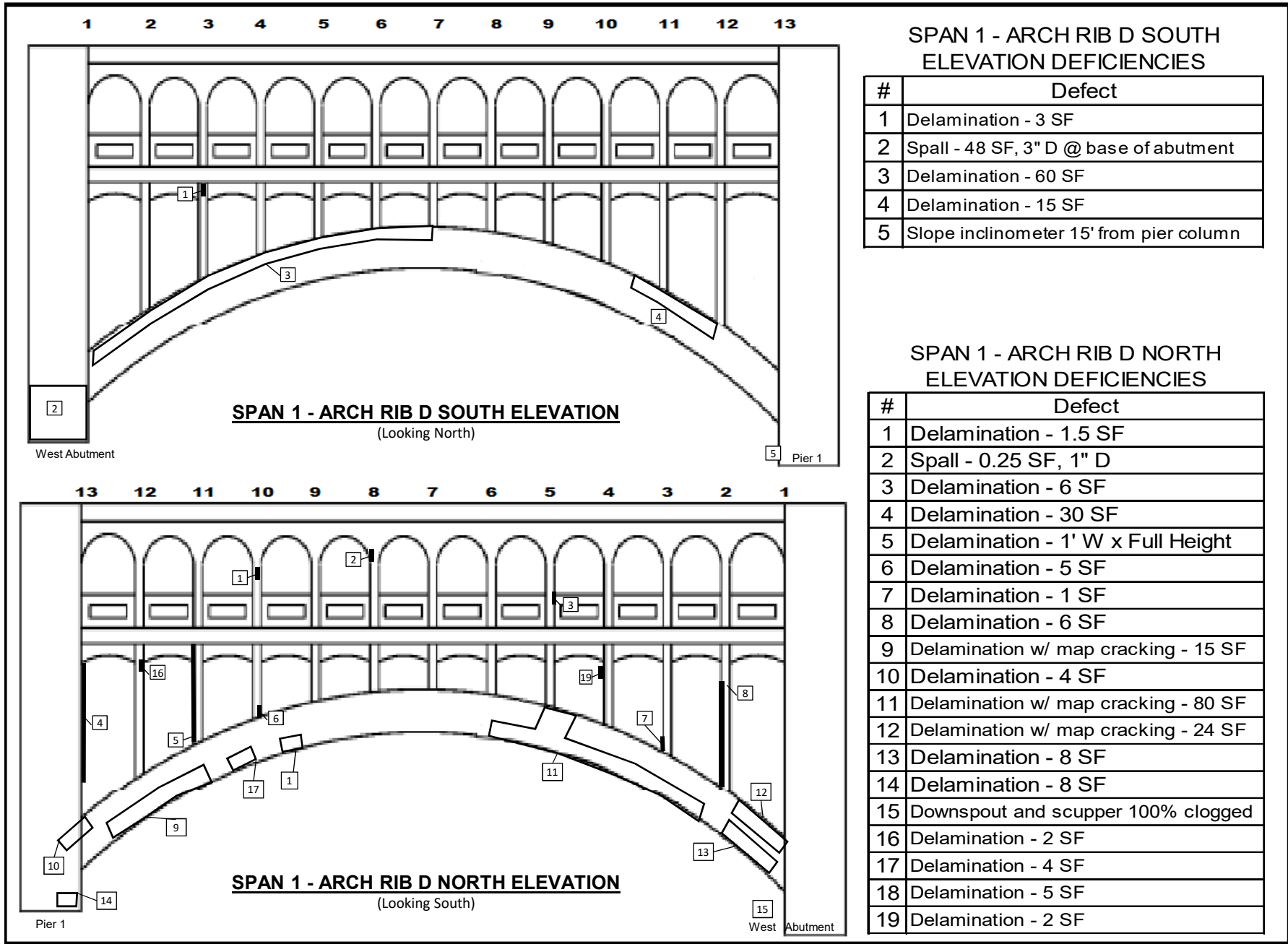
**SPAN 1B - COLUMN LINE C
NORTH ELEVATION**

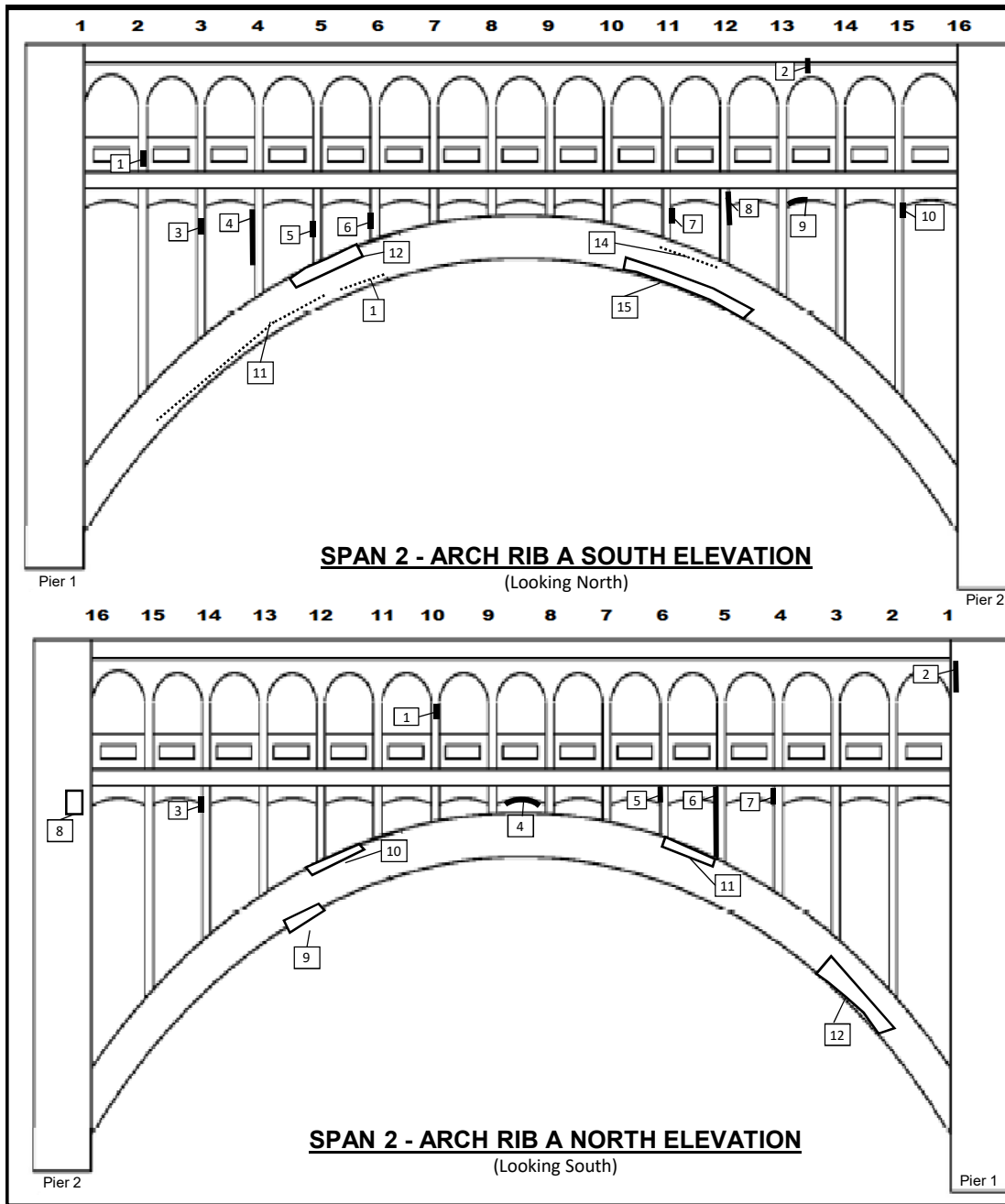






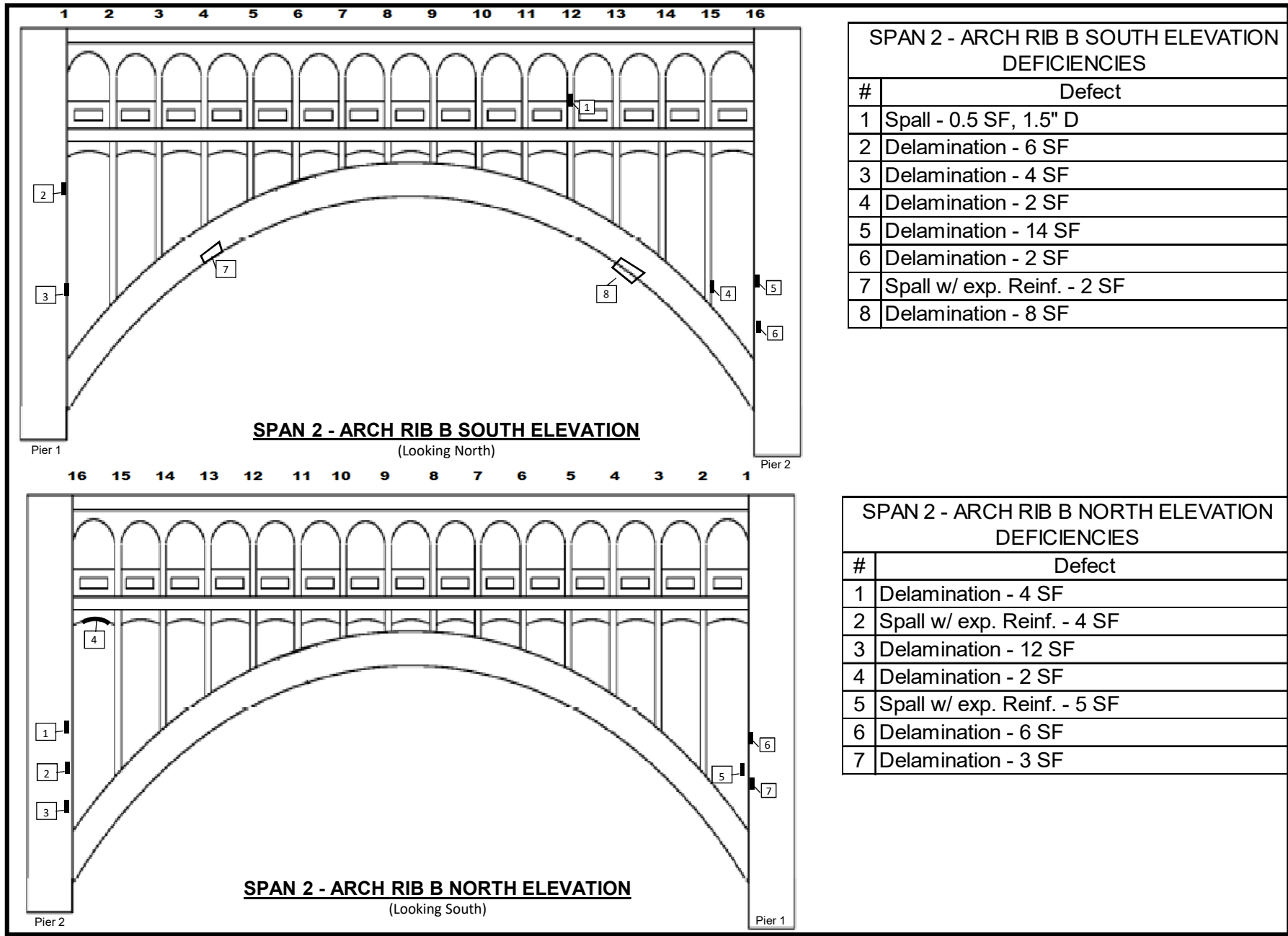






RIB A SOUTH ELEV. DEFICIENCIES	
#	Defect
1	Delamination - 3 SF
2	Delamination - 5 SF
3	Spall w/ exp. Reinf. - 6 SF
4	Spall w/ exp. Reinf. - 14 SF
5	Delamination - 2 SF
6	Delamination - 6 SF
7	Delamination - 1 SF
8	Spall w/ exp. Reinf. - 8 SF
9	Delamination w/ Spall w/ exp. Reinf. - 12 SF
10	Delamination - 3 SF
11	25' L x up to 1/8" W crack
12	Delamination - 8 SF
13	5' L x 1/16" W crack
14	15' L crack w/ efflorescence
15	Delamination - 28 SF

RIB A NORTH ELEV. DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 1 SF
2	Delamination - 4 SF (on pier tower)
3	Delamination - 4 SF
4	Delamination - 5 SF
5	Delamination - 2 SF
6	Delamination - 4 SF
7	Delamination - 6 SF
8	Delamination - 2 SF
9	Delamination - 4 SF (underside of arch)
10	Delamination - 6 SF
11	Delamination - 3 SF
12	Delamination w/ Spall - 10 SF

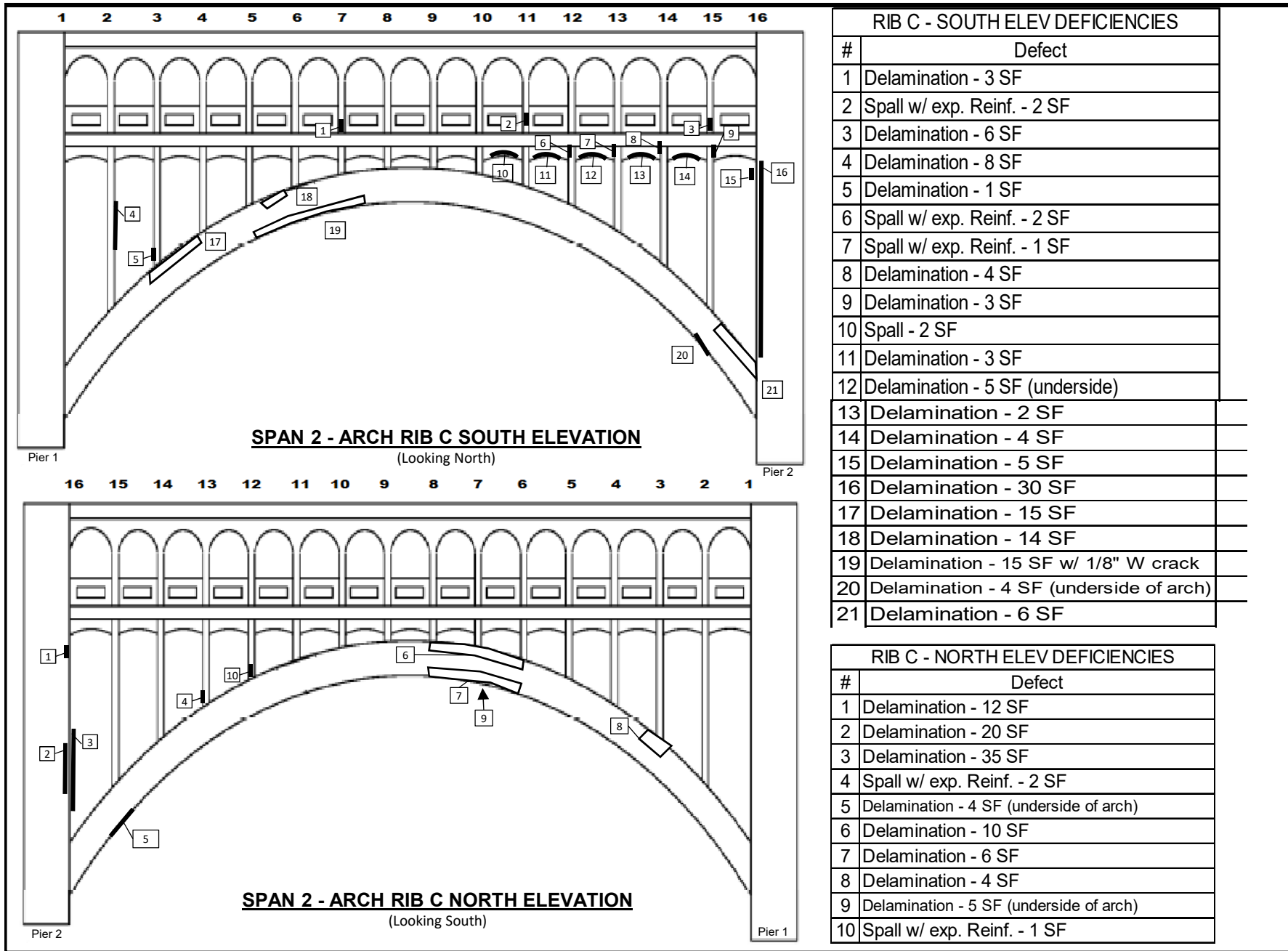


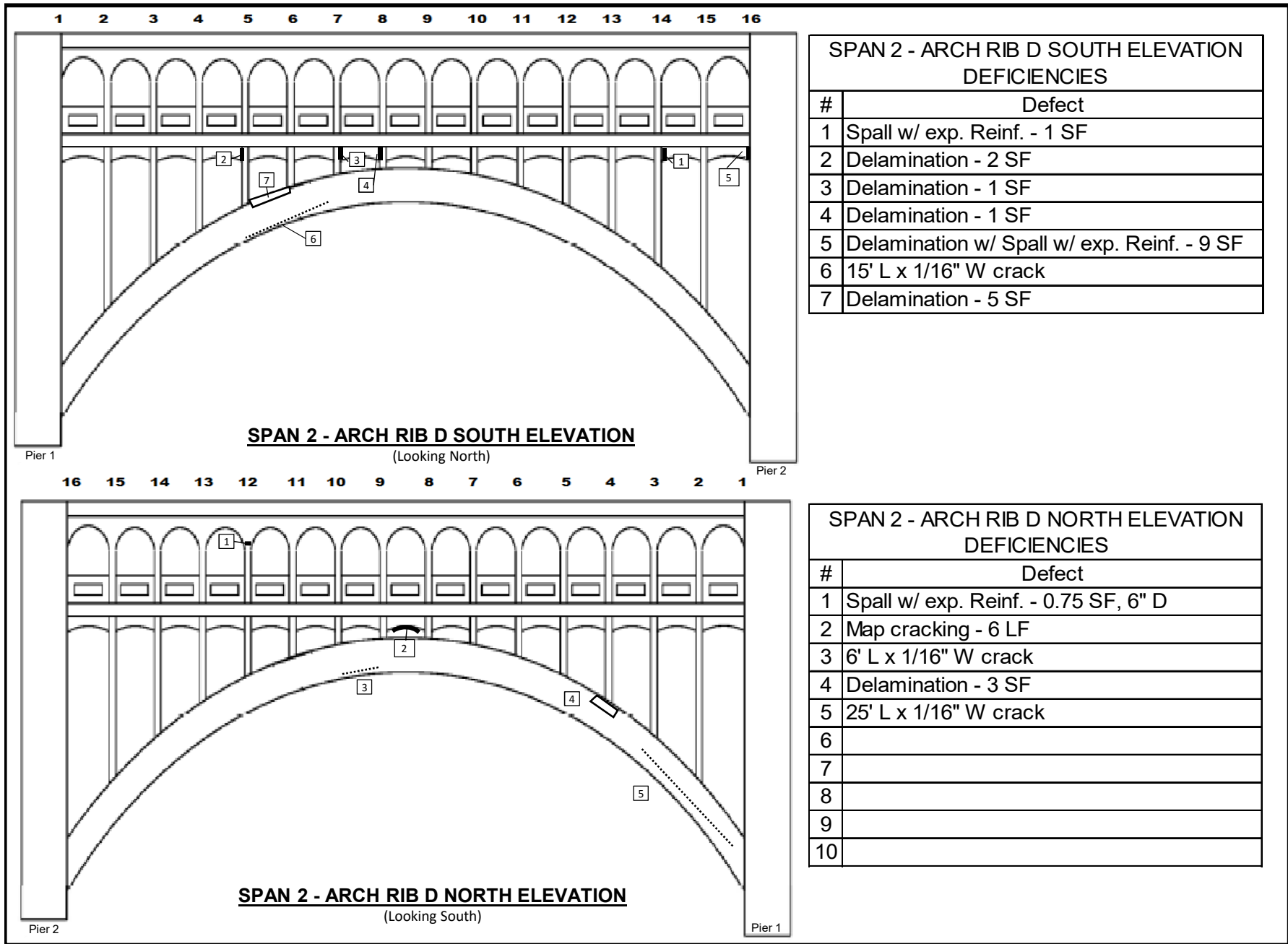
SPAN 2 - ARCH RIB B SOUTH ELEVATION DEFICIENCIES

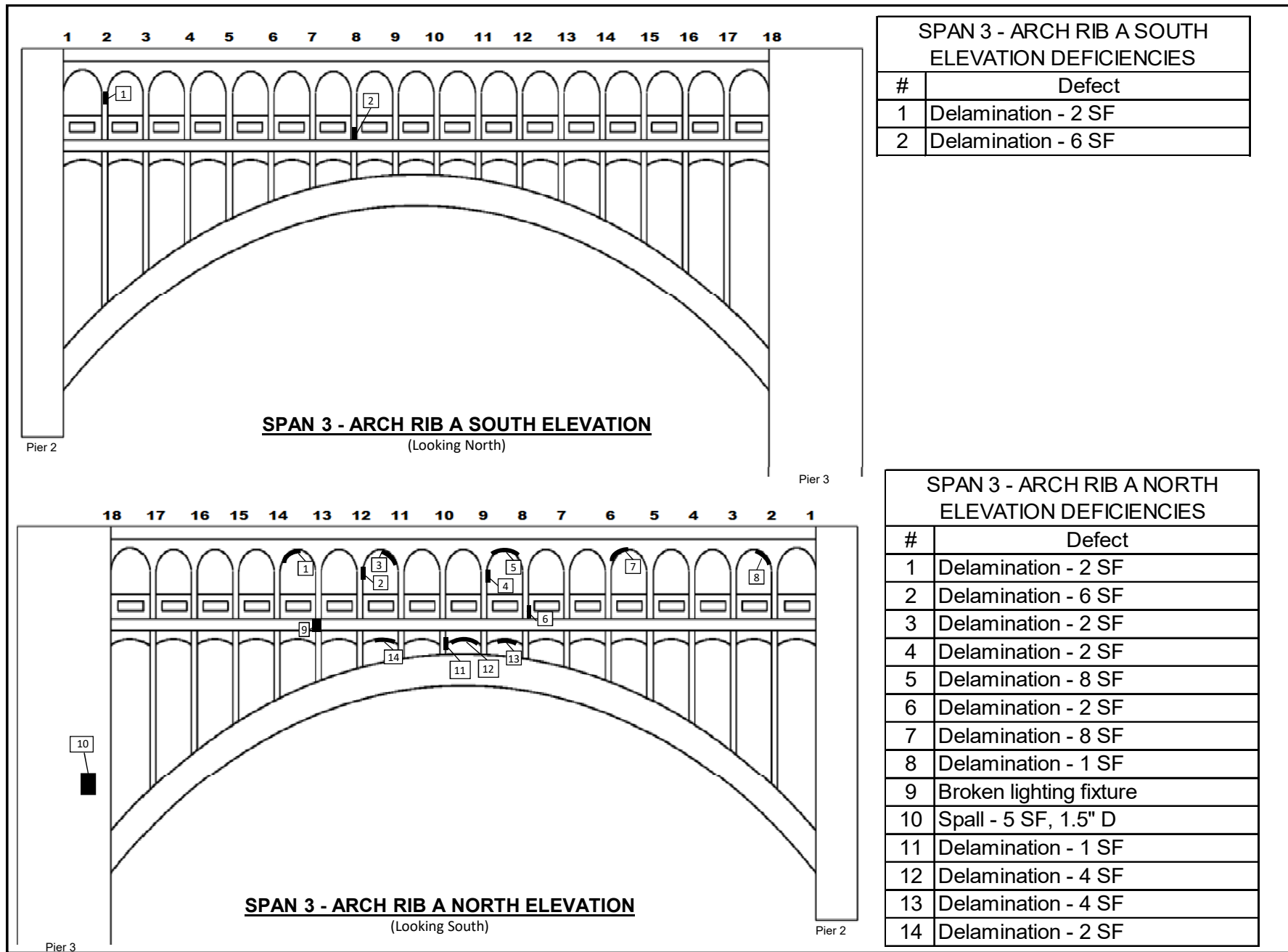
#	Defect
1	Spall - 0.5 SF, 1.5" D
2	Delamination - 6 SF
3	Delamination - 4 SF
4	Delamination - 2 SF
5	Delamination - 14 SF
6	Delamination - 2 SF
7	Spall w/ exp. Reinf. - 2 SF
8	Delamination - 8 SF

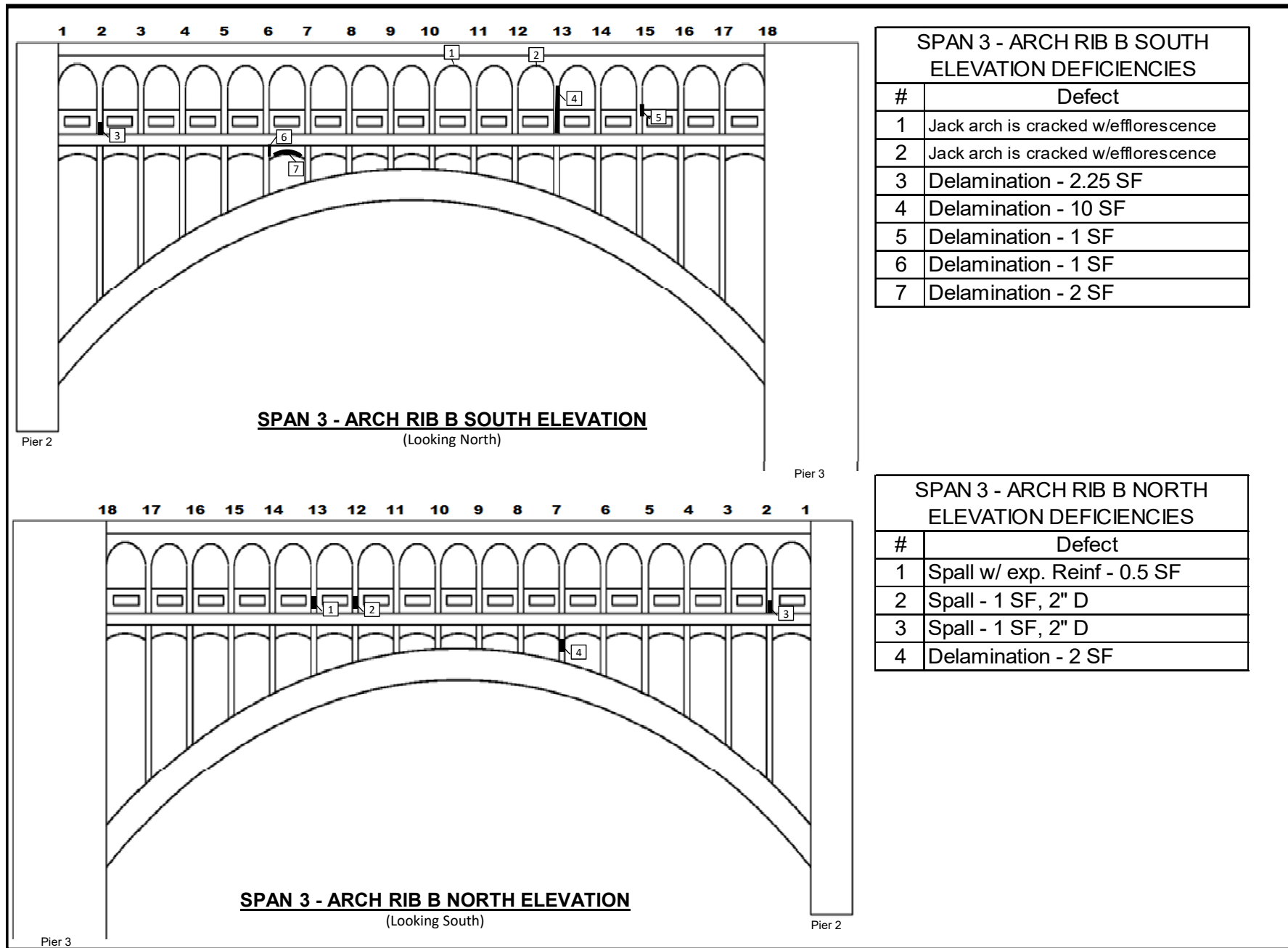
SPAN 2 - ARCH RIB B NORTH ELEVATION DEFICIENCIES

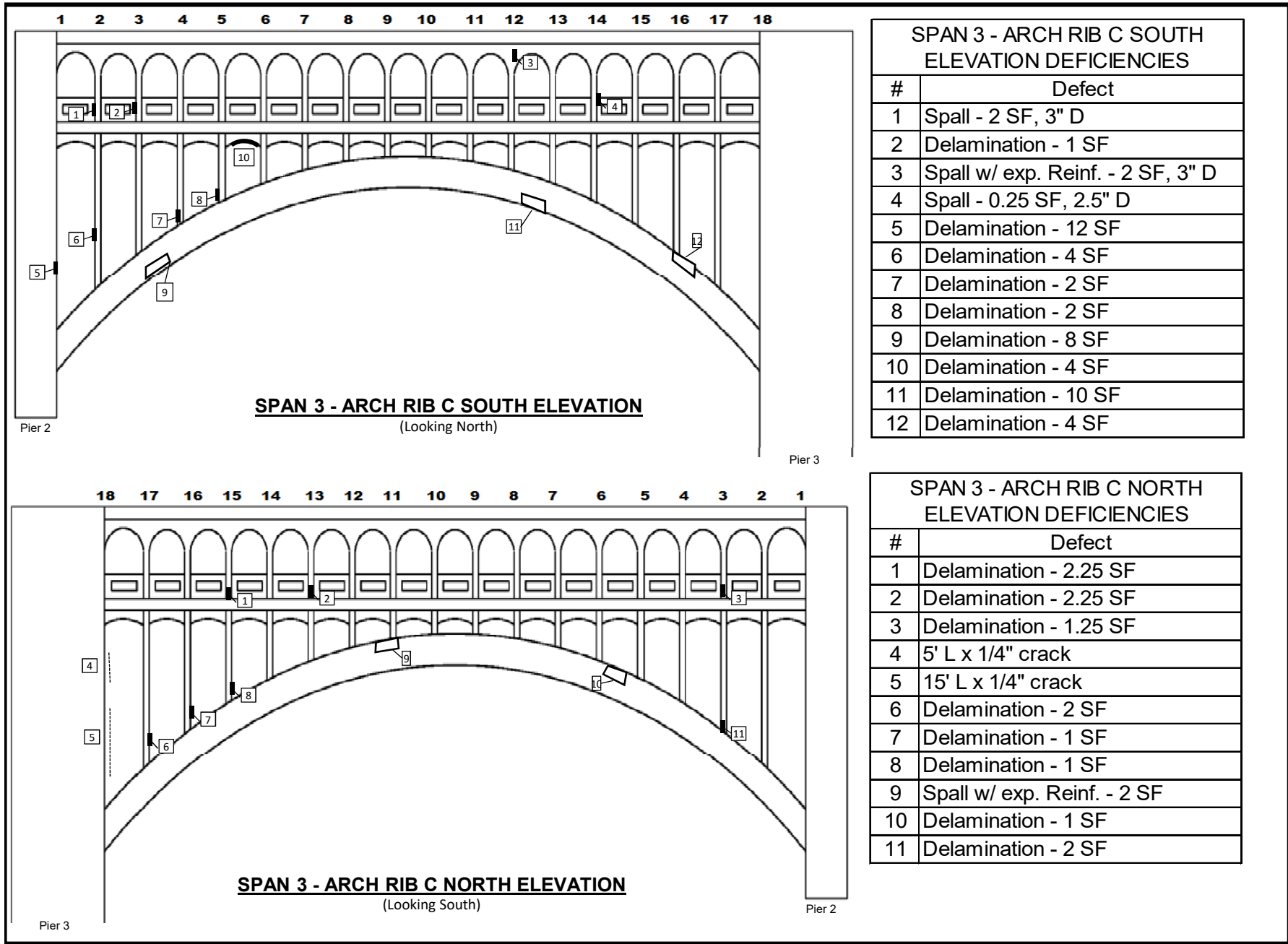
#	Defect
1	Delamination - 4 SF
2	Spall w/ exp. Reinf. - 4 SF
3	Delamination - 12 SF
4	Delamination - 2 SF
5	Spall w/ exp. Reinf. - 5 SF
6	Delamination - 6 SF
7	Delamination - 3 SF

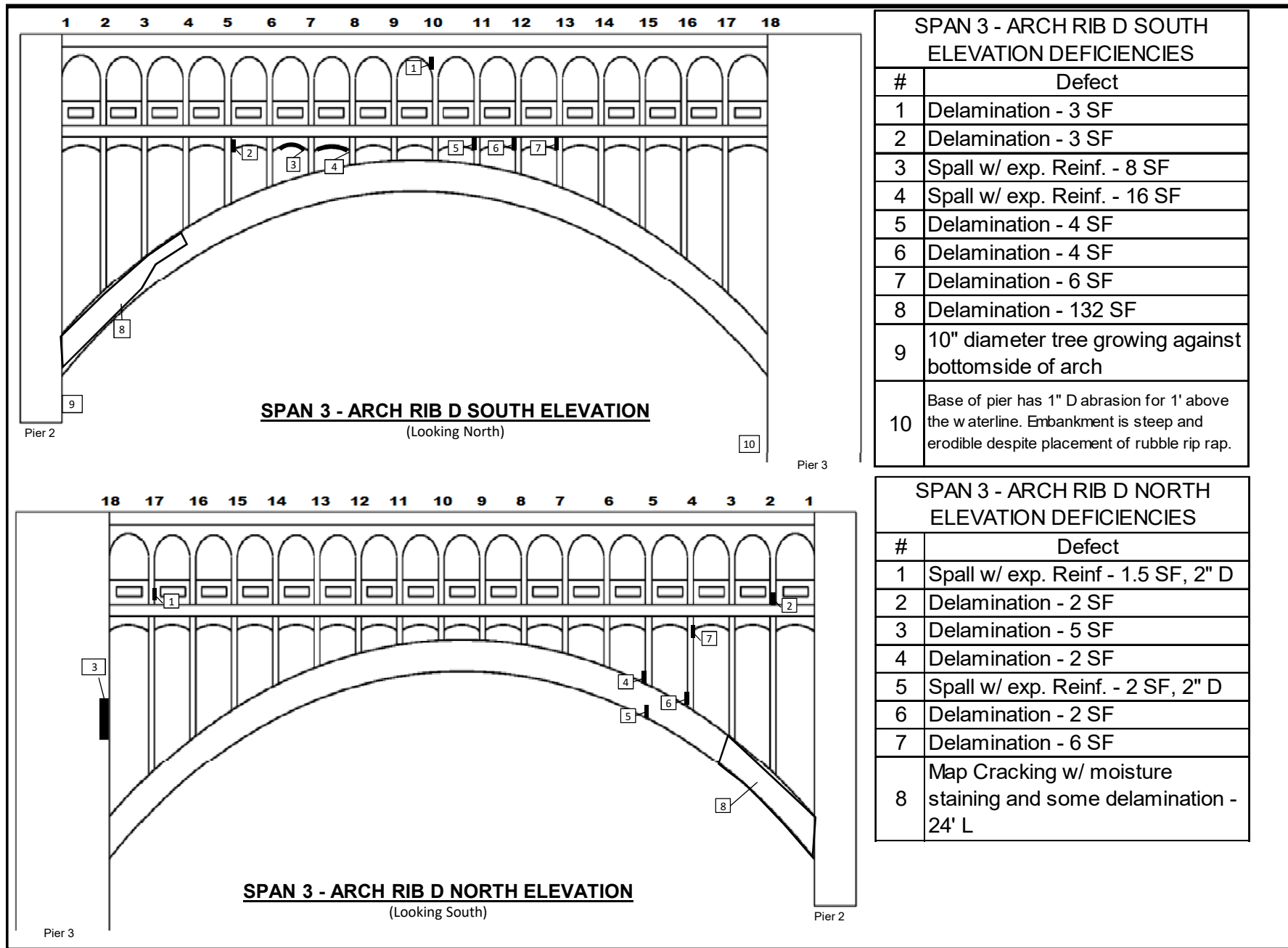


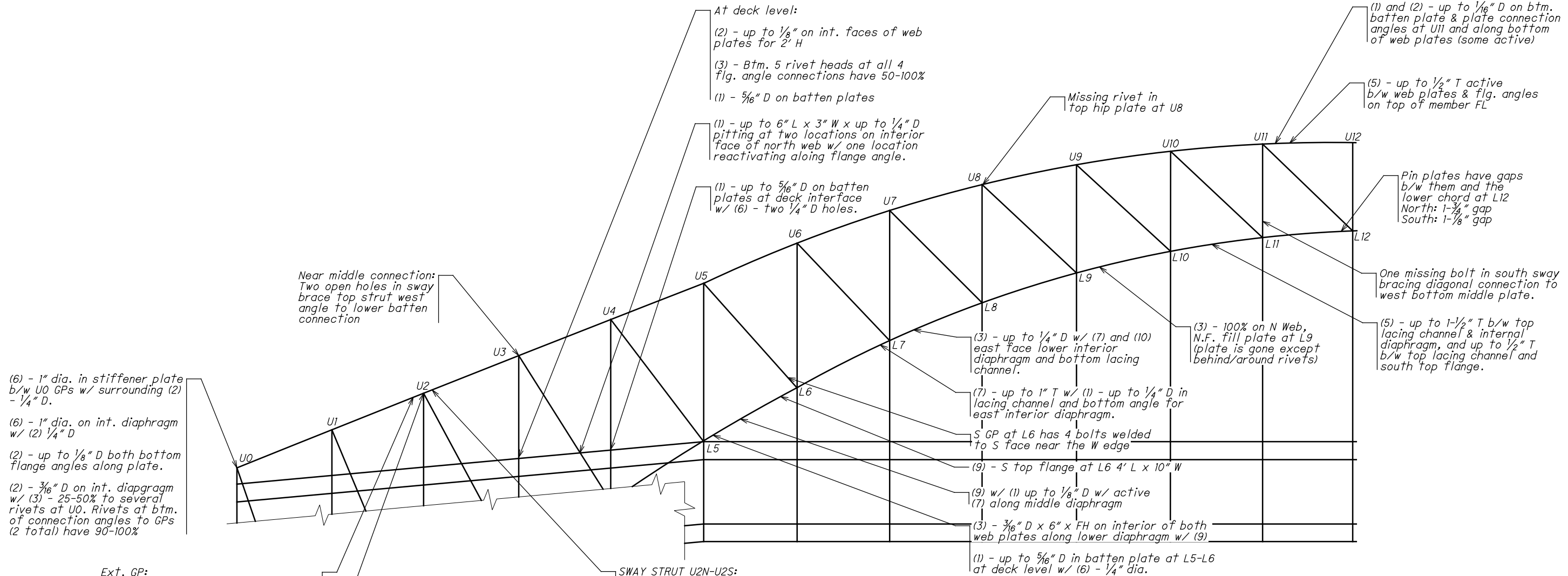










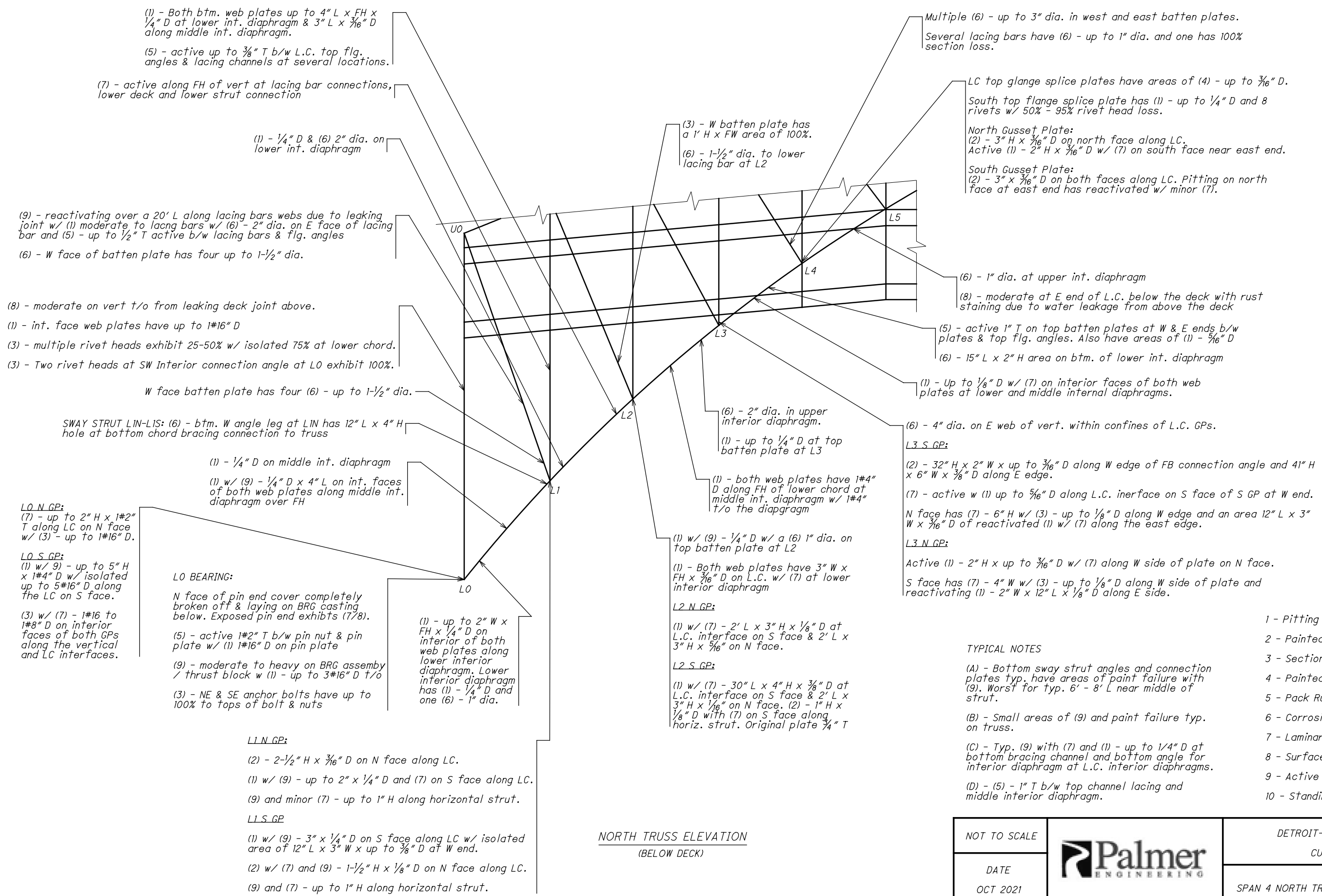


NORTH TRUSS ELEVATION
(ABOVE DECK)

TYPICAL NOTES

- (A) - Bottom sway strut angles and connection plates typ. have areas of paint failure with (9). Worst for typ. 6' - 8' L near middle of strut.
 - (B) - Small areas of (9) and paint failure typ. on truss.
 - (C) - Typ. (9) with (7) and (1) - up to 1/4" D at bottom bracing channel and bottom angle for interior diaphragm at L.C. interior diaphragms.
 - (D) - (5) - 1" T b/w top channel lacing and middle interior diaphragm.
 - (E) - Isolated lacing channels have minor impact deformations in their flanges.
 - (F) - Isolated active (5) - up to 3/8" b/w sway bracing components and/or connections.
- 1 - Pitting
 - 2 - Painted Over Pitting
 - 3 - Section Loss
 - 4 - Painted Over Section Loss
 - 5 - Pack Rust
 - 6 - Corrosion Hole / Perforation
 - 7 - Laminar Corrosion
 - 8 - Surface Corrosion
 - 9 - Active Corrosion
 - 10 - Standing Water

NOT TO SCALE		DETROIT-SUPERIOR BRIDGE	
DATE OCT 2021		CUY-6-14.56	
		SPAN 4 NORTH TRUSS ELEVATION	PAGE C-1



(1) - Both btm. web plates up to 4" L x FH x 1/4" D at lower int. diaphragm & 3" L x 3/16" D along middle int. diaphragm.

(5) - active up to 3/8" T b/w L.C. top flg. angles & lacing channels at several locations.

(7) - active along FH of vert at lacing bar connections, lower deck and lower strut connection

(1) - 1/4" D & (6) 2" dia. on lower int. diaphragm

(9) - reactivating over a 20' L along lacing bars webs due to leaking joint w/ (1) moderate to lacng bars w/ (6) - 2" dia. on E face of lacing bar and (5) - up to 1/2" T active b/w lacing bars & flg. angles

(6) - W face of batten plate has four up to 1-1/2" dia.

(8) - moderate on vert t/o from leaking deck joint above.

(1) - int. face web plates have up to 1#16" D

(3) - multiple rivet heads exhibit 25-50% w/ isolated 75% at lower chord.

(3) - Two rivet heads at SW Interior connection angle at L0 exhibit 100%.

W face batten plate has four (6) - up to 1-1/2" dia.

SWAY STRUT LIN-LIS: (6) - btm. W angle leg at LIN has 12" L x 4" H hole at bottom chord bracing connection to truss

(1) - 1/4" D on middle int. diaphragm

(1) w/ (9) - 1/4" D x 4" L on int. faces of both web plates along middle int. diaphragm over FH

L0 N GP:
(7) - up to 2" H x 1#2" T along LC on N face w/ (3) - up to 1#16" D.

L0 S GP:
(1) w/ (9) - up to 5" H x 1#4" D w/ isolated up to 5#16" D along the LC on S face.

(3) w/ (7) - 1#16 to 1#8" D on interior faces of both GPs along the vertical and LC interfaces.

L0 BEARING:

N face of pin end cover completely broken off & laying on BRG casting below. Exposed pin end exhibits (7/8).

(5) - active 1#2" T b/w pin nut & pin plate w/ (1) 1#16" D on pin plate

(9) - moderate to heavy on BRG assembly / thrust block w (1) - up to 3#16" D t/o

(3) - NE & SE anchor bolts have up to 100% to tops of bolt & nuts

L1 N GP:

(2) - 2-1/2" H x 3/16" D on N face along LC.

(1) w/ (9) - up to 2" x 1/4" D and (7) on S face along LC.

(9) and minor (7) - up to 1" H along horizontal strut.

L1 S GP:

(1) w/ (9) - 3" x 1/4" D on S face along LC w/ isolated area of 12" L x 3" W x up to 3/8" D at W end.

(2) w/ (7) and (9) - 1-1/2" H x 1/8" D on N face along LC.

(9) and (7) - up to 1" H along horizontal strut.

(1) - up to 2" W x FH x 1/4" D on interior of both web plates along lower interior diaphragm. Lower interior diaphragm has (1) - 1/4" D and one (6) - 1" dia.

(3) - W batten plate has a 1' H x FW area of 100%.

(6) - 1-1/2" dia. to lower lacing bar at L2

(6) - 2" dia. in upper interior diaphragm.

(1) - up to 1/4" D at top batten plate at L3

(1) - both web plates have 1#4" D along FH of lower chord at middle int. diaphragm w/ 1#4" t/o the diaphragm

(1) w/ (9) - 1/4" D w/ a (6) 1" dia. on top batten plate at L2

(1) - Both web plates have 3" W x FH x 3/16" D on L.C. w/ (7) at lower interior diaphragm

L2 N GP:

(1) w/ (7) - 2' L x 3" H x 1/8" D at L.C. interface on S face & 2' L x 3" H x 3/16" on N face.

L2 S GP:

(1) w/ (7) - 30" L x 4" H x 3/8" D at L.C. interface on S face & 2' L x 3" H x 1/16" on N face. (2) - 1" H x 1/8" D with (7) on S face along horiz. strut. Original plate 3/4" T

Multiple (6) - up to 3" dia. in west and east batten plates.
Several lacing bars have (6) - up to 1" dia. and one has 100% section loss.

LC top glange splice plates have areas of (4) - up to 3/16" D.
South top flange splice plate has (1) - up to 1/4" D and 8 rivets w/ 50% - 95% rivet head loss.

North Gusset Plate:
(2) - 3" H x 3/16" D on north face along LC.
Active (1) - 2" H x 3/16" D w/ (7) on south face near east end.

South Gusset Plate:
(2) - 3" x 3/16" D on both faces along LC. Pitting on north face at east end has reactivated w/ minor (7).

(6) - 1" dia. at upper int. diaphragm

(8) - moderate at E end of L.C. below the deck with rust staining due to water leakage from above the deck

(5) - active 1" T on top batten plates at W & E ends b/w plates & top flg. angles. Also have areas of (1) - 3/16" D

(6) - 15" L x 2" H area on btm. of lower int. diaphragm

(1) - Up to 1/8" D w/ (7) on interior faces of both web plates at lower and middle internal diaphragms.

(6) - 4" dia. on E web of vert. within confines of L.C. GPs.

L3 S GP:

(2) - 32" H x 2" W x up to 3/16" D along W edge of FB connection angle and 41" H x 6" W x 3/8" D along E edge.

(7) - active w (1) up to 5/16" D along L.C. interface on S face of S GP at W end.
N face has (7) - 6" H w/ (3) - up to 1/8" D along W edge and an area 12" L x 3" W x 3/16" D of reactivated (1) w/ (7) along the east edge.

L3 N GP:

Active (1) - 2" H x up to 3/16" D w/ (7) along W side of plate on N face.

S face has (7) - 4" W w/ (3) - up to 1/8" D along W side of plate and reactivating (1) - 2" W x 12" L x 1/8" D along E side.

TYPICAL NOTES

(A) - Bottom sway strut angles and connection plates typ. have areas of paint failure with (9). Worst for typ. 6" - 8" L near middle of strut.

(B) - Small areas of (9) and paint failure typ. on truss.

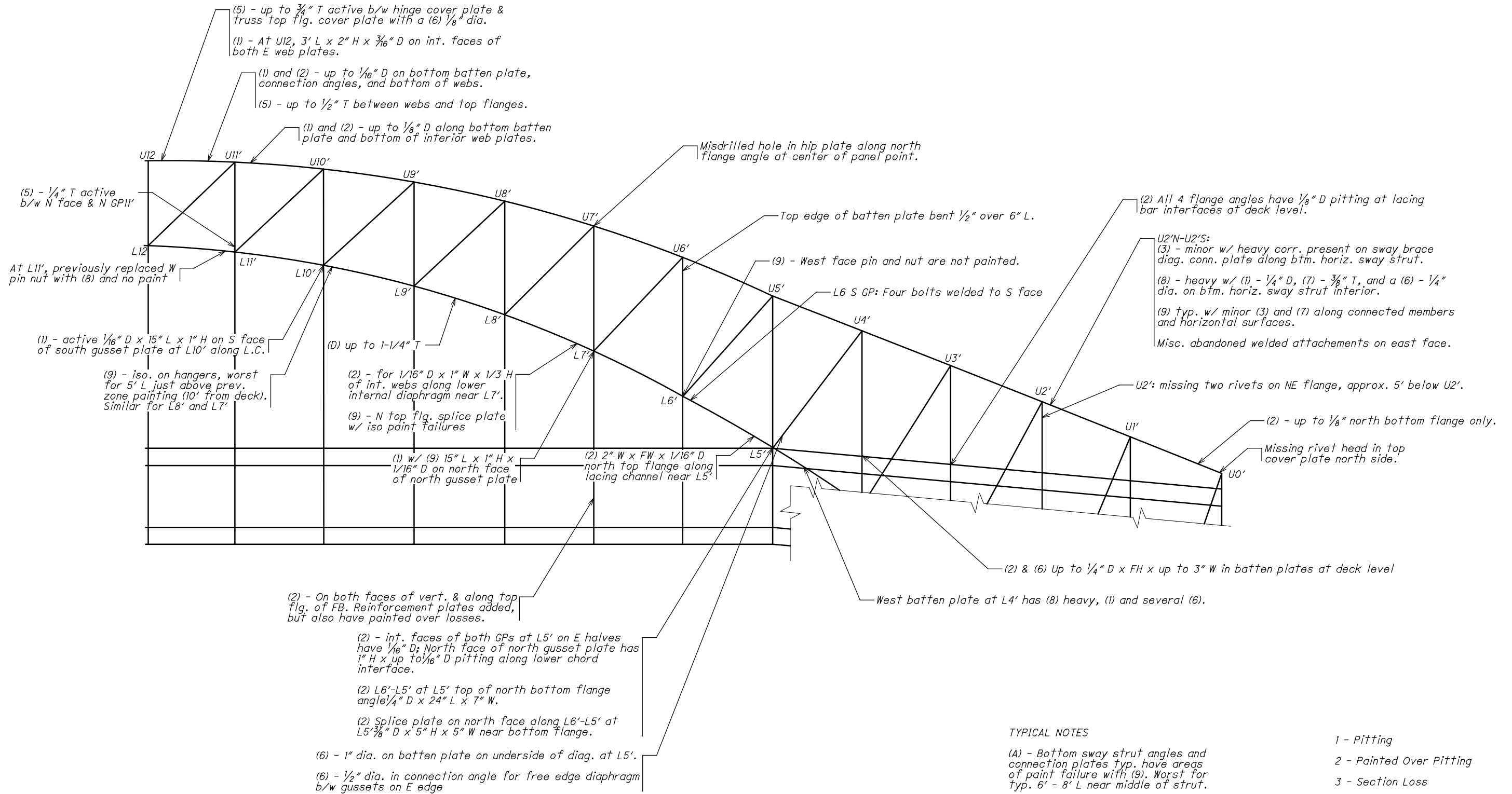
(C) - Typ. (9) with (7) and (1) - up to 1/4" D at bottom bracing channel and bottom angle for interior diaphragm at L.C. interior diaphragms.

(D) - (5) - 1" T b/w top channel lacing and middle interior diaphragm.

- 1 - Pitting
- 2 - Painted Over Pitting
- 3 - Section Loss
- 4 - Painted Over Section Loss
- 5 - Pack Rust
- 6 - Corrosion Hole / Perforation
- 7 - Laminar Corrosion
- 8 - Surface Corrosion
- 9 - Active Corrosion
- 10 - Standing Water

NORTH TRUSS ELEVATION
(BELOW DECK)

NOT TO SCALE		DETROIT-SUPERIOR BRIDGE	
DATE OCT 2021		CUY-6-14.56	
		SPAN 4 NORTH TRUSS ELEVATION	PAGE C-2

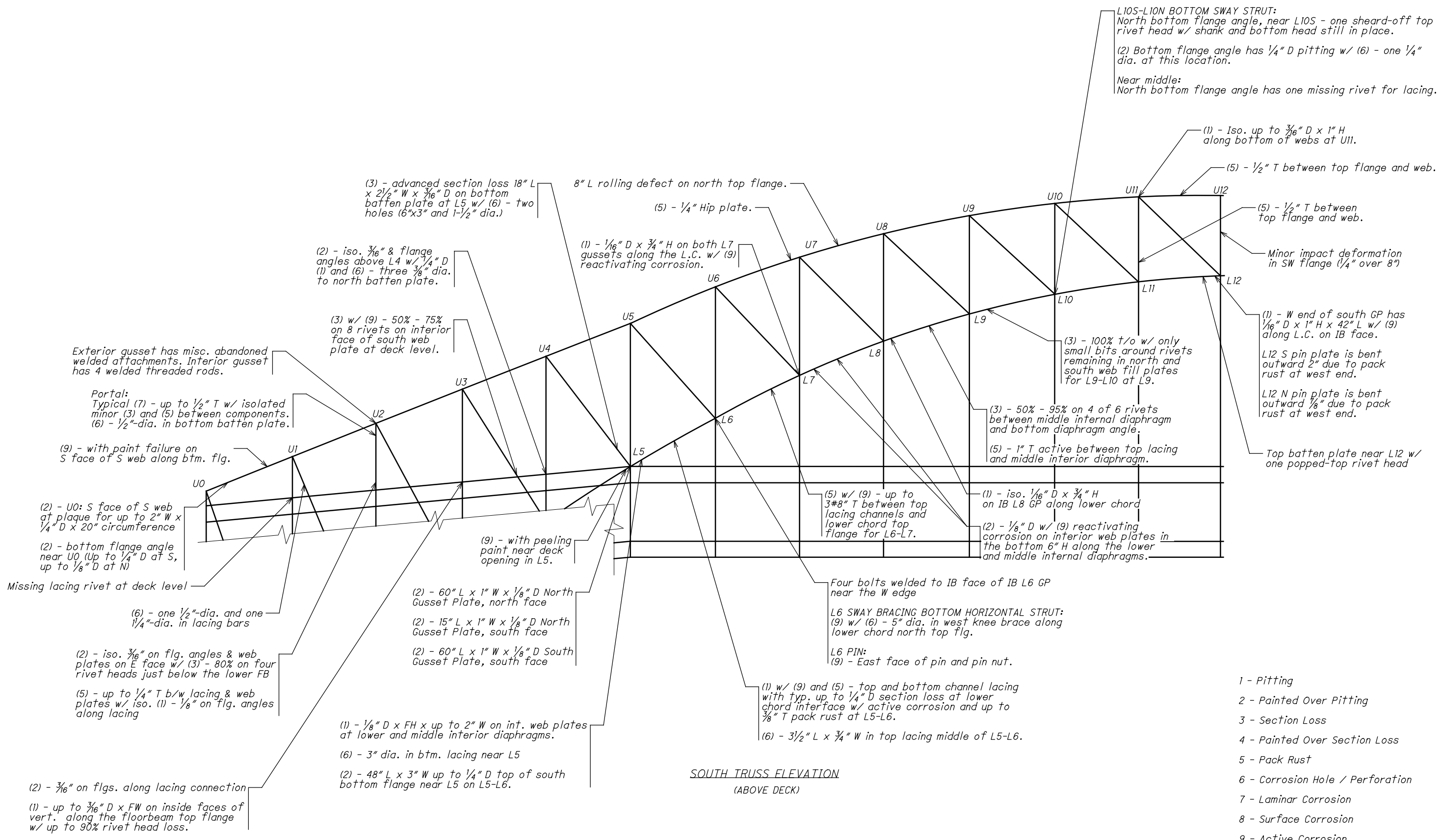


NORTH TRUSS ELEVATION
(ABOVE DECK)

TYPICAL NOTES

- (A) - Bottom sway strut angles and connection plates typ. have areas of paint failure with (9). Worst for typ. 6' - 8' L near middle of strut.
 - (B) - Small areas of (9) and paint failure typ. on truss.
 - (C) - Typ. (9) with (7) and (1) - up to 1/4" D at bottom bracing channel and bottom angle for interior diaphragm at L.C. interior diaphragms.
 - (D) - (5) - 1" T b/w top channel lacing and middle interior diaphragm.
- 1 - Pitting
 - 2 - Painted Over Pitting
 - 3 - Section Loss
 - 4 - Painted Over Section Loss
 - 5 - Pack Rust
 - 6 - Corrosion Hole / Perforation
 - 7 - Laminar Corrosion
 - 8 - Surface Corrosion
 - 9 - Active Corrosion

NOT TO SCALE		DETROIT-SUPERIOR BRIDGE	
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		SPAN 4 NORTH TRUSS ELEVATION	PAGE C-3



SOUTH TRUSS ELEVATION
(ABOVE DECK)

- 1 - Pitting
- 2 - Painted Over Pitting
- 3 - Section Loss
- 4 - Painted Over Section Loss
- 5 - Pack Rust
- 6 - Corrosion Hole / Perforation
- 7 - Laminar Corrosion
- 8 - Surface Corrosion
- 9 - Active Corrosion

- TYPICAL NOTES**
- Uphill transverse angles have debris causing pitting and perforations of the diaphragm plates and transverse angles on lower chord between L0 to L5.
 - Typical small locations of paint failure and active surface corrosion throughout with areas of minor ($1/16''$ D) section loss along connecting elements.
 - Typical active corrosion and up to $1/4''$ D pitting on interior diaphragm and angles for lower chord members.

NOT TO SCALE		DETROIT-SUPERIOR BRIDGE	
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		SPAN 4 SOUTH TRUSS ELEVATION	PAGE C-5

J:\ODOT\109534_VAR-D12 Inspections\Inspection\CUY-6-14-56_Detroit Superior_1800930_2021Report\CADD\Span 4_Span 4_South Truss - West End - Below Deck 1/25/2022 10:43:59 AM adam-l

(3) - 100% loss on several north face lacing bars.
 (2) - iso. $\frac{3}{16}$ " on flg. angles & web plates on E face w/
 (3) - 80% on four rivet heads just below the lower FB
 (5) - up to $\frac{1}{4}$ " T b/w lacing & web plates w/ iso. (1) - $\frac{1}{8}$ " on flg. angles along lacing

(9) - w/ peeling paint and failed paint near top
 (5) - up to $\frac{3}{4}$ " T b/w flg. angles & lacing with lacing deformed
 (1) - isolated $\frac{1}{8}$ " to $\frac{3}{16}$ " along flg. angles at iso. lacing bars

Vertical has (8 & 9)
 (5) - up to $\frac{1}{4}$ " T b/w webs & flanges. $\frac{3}{4}$ " at lacing bars w/ iso $\frac{1}{8}$ " D pitting in flg. angles along the lacing
 (1) - isolated $\frac{1}{8}$ " D on the interior web plates (worst near L0)

(1) - Up to $\frac{1}{8}$ " D on flanges and webs.
 (5) - Up to $\frac{5}{8}$ " T between lacing and flanges.

(6) - second top transv. lacing channel from L0 has a 5" H x $\frac{3}{4}$ " W and a $\frac{1}{2}$ " dia. Fourth lacing channel from L0 has a 6" L x 2" W

L0 IB PIN PLATE:
 (SEE SKETCH)
 There is a $5\frac{3}{4}$ " L crack with a $4\frac{3}{4}$ " L crack propagates off of the other crack, which extends to the end of the plate. There is also a $3\frac{3}{8}$ " L crack extending from the nut at 10 o'clock, and a new $3\frac{1}{2}$ " L paint crack extending from the nut at 1:30.

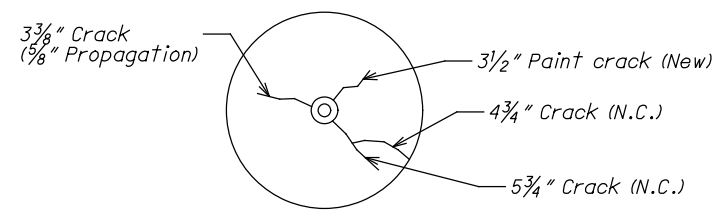
L0 OB PIN PLATE:
 has a 7" L vertical crack extending upward from the nut and a $1\frac{3}{4}$ " L vertical crack extending downward from the nut. This crack may be continuing beyond the current end point, but unable to verify due to extensive surface corrosion.

BEARING:
 (8) - moderate to advanced t/o thrust block w/ peeling paint and (7).
 (5) - up to 1" T b/w BRG behind pin & L.C.
 (3) - 3 of 6 AB nuts in thrust block have 90% (other bolts have 10%)

L0 N Gusset:
 8 rivet heads w/ (3) - up to 90% on U0-L0 connection.
 North face - (7) w/ (1) - up to $\frac{1}{8}$ " D x 6" H x 45" L along lower chord.
 South face - isolated (1) - $\frac{1}{16}$ " D along vertical.

L0 S gusset:
 South face - (7) w/ (1) - up to $\frac{1}{8}$ " D x 3" H x 60" L along lower chord.
 North face - (1) - up to $\frac{1}{4}$ " D x 2" H x 30" L along diaphragm, and $\frac{3}{16}$ " D x 6" H x 2" W along vertical.

Sheared bolt on top 3rd lacing channel connection along top flg. Also a fractured bolt in end of batten plate at L0.
 (6) - Four small holes up to 1" dia in diaphragm near L1.



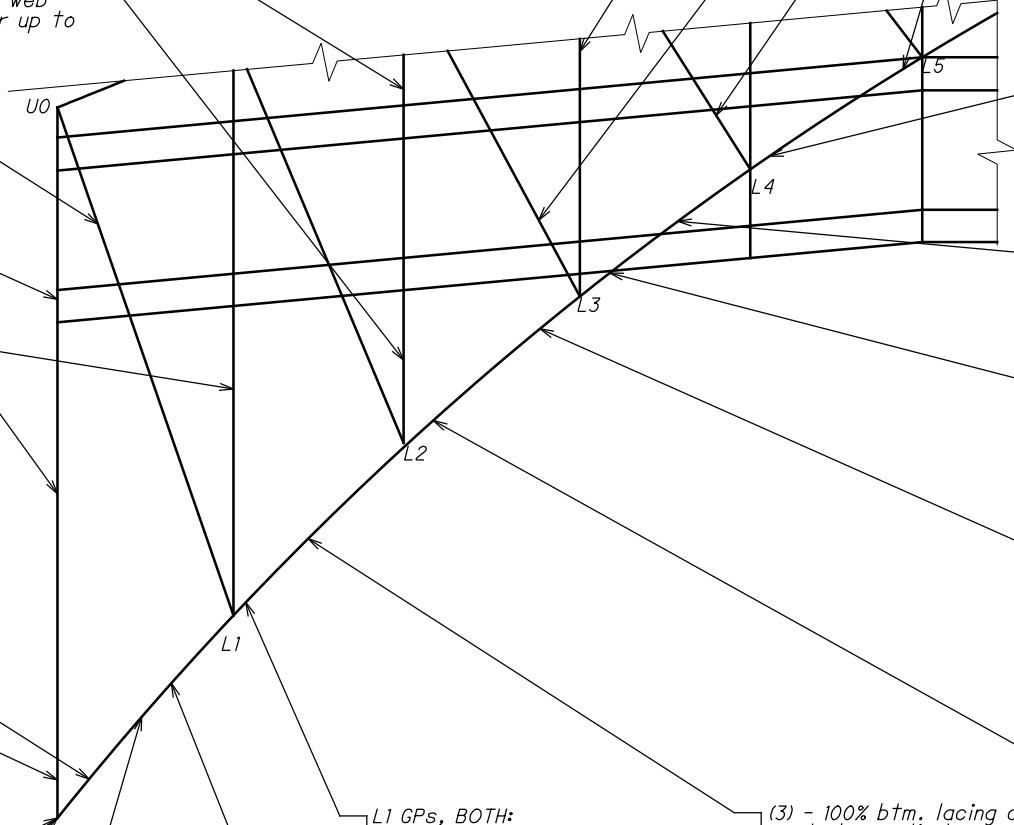
South Truss L0 Bearing
 North Pin Cover Plate
 (Not to scale)

Cantilever brace:
 (1) - $\frac{1}{4}$ " D in web w/ (6) - four up to 2"-dia.

West sway brace connection plate bowed at strut up to $\frac{5}{8}$ " due to pack rust.

Sway strut has (9) throughout interior faces.

Top later brace connection plate w/ impact dent $1\frac{1}{4}$ " over 10".



(2) - $\frac{3}{16}$ " on flgs. along lacing
 (6) - Three 6" dia. on top batten plate.
 (1) - up to $\frac{3}{16}$ " D x FW on inside faces of vert. along top of the lower FB w/ up to 90% rivet head loss.

(6) - Two 2"-dia. in lower batten plate.

(6) - numerous holes t/o btm. batten plate at L4.

(9) - with peeling paint near deck opening in L5.
 (1) - $\frac{1}{8}$ " D on int. web plates
 (6) - 3" dia. in btm. lacing near L5

L4 GPs, BOTH:
 (2) - up to $\frac{3}{16}$ " x 2" H x FL on ext. face along L.C., iso. $\frac{1}{4}$ " D x 2" H x 4" L on int. face along L3L4.
 Isolated $\frac{1}{4}$ " D x 2" H x 10" L on int. faces along L4L5.
 Six plug welds, 3 on each half, on each gusset plate.
 (2) - $\frac{1}{4}$ " D on north top flange splice plate.

(1) - $\frac{5}{16}$ " D x FH, N int. web plate $\frac{3}{16}$ " D x 12" H on S int. web plate
 (5) - up to 1" T b/w batten plates & top flange
 (1) - up to $\frac{3}{16}$ " D on top flg. angles along lacing.

L3, BOTH GPs:
 (1) - up to $\frac{3}{16}$ " D along lower chords on ext. faces and (8). (1) - $\frac{3}{16}$ " avg. on int. faces except S face of N plate along L3-L4, which is $\frac{5}{16}$ " ($\frac{1}{4}$ " remaining).
L3, IB GP has:
 (2) - three areas of up to $\frac{1}{4}$ " D x 3" W x 8" H on N face along W face of the vertical.
 (6) - East face stiffening diaphragm has a 4" dia.
 (1) - Up to $\frac{1}{4}$ " D south top flange splice

(1) - N int. web plate has 4" L x Iso. FH x $\frac{1}{16}$ " D active along int. diaphragm near L2. S int. web plate has $\frac{1}{4}$ " D at this location.
 (5) - up to $\frac{5}{8}$ " T b/w top batten plates & flanges

L1 GPs, BOTH:

(7) - FL x 3" H x $\frac{1}{4}$ " D of LOL1 along struts on ext. faces. (1) - 12" L x 2" H x $\frac{1}{8}$ " D on inside faces along LOL1.
 (2) - 3" H x $\frac{1}{4}$ " D along L1L2 on ext. faces and up to 12" on vert. Inside faces along L1-L2 have (1) - ($\frac{1}{8}$ " on N, $\frac{1}{4}$ " on S)
 (2) - $\frac{1}{4}$ " on south top flange splice plate.
 (6) - 2"-dia. on edge stiffening diaphragm on east end.
 (1) - $\frac{1}{4}$ " D x 4" L x FH in webs along interior diaphragm.

(3) - 100% btm. lacing channel vert. legs adj. to L.C. web plates

(3) - top batten plate at L1 has 12" W x 3" L x 100% along bottom edge, (6) - 1" dia. and advanced s.l. t/o.

(1) - along int. diaphragms: N int plate: 3" L x FH x $\frac{1}{4}$ " D S int plate: 4" L x $\frac{1}{2}$ " H x $\frac{1}{4}$ " D
 (6) - two up to 2"-dia. in interior diaphragm
 (2) - up to $\frac{1}{8}$ " on web plates

L2 IB (N) GP:

(1) - $\frac{1}{8}$ " D top of diagonal, S face
 (1) - above L1-L2 has 24" L x 2" H x $\frac{3}{16}$ " D on S face 30" L x 3" H x $\frac{1}{4}$ " D on N face
 (1) - above L2-L3 has 24" L x 2" H x $\frac{1}{4}$ " D on both faces ($\frac{1}{4}$ " remaining)

L2 OB (S) GP:

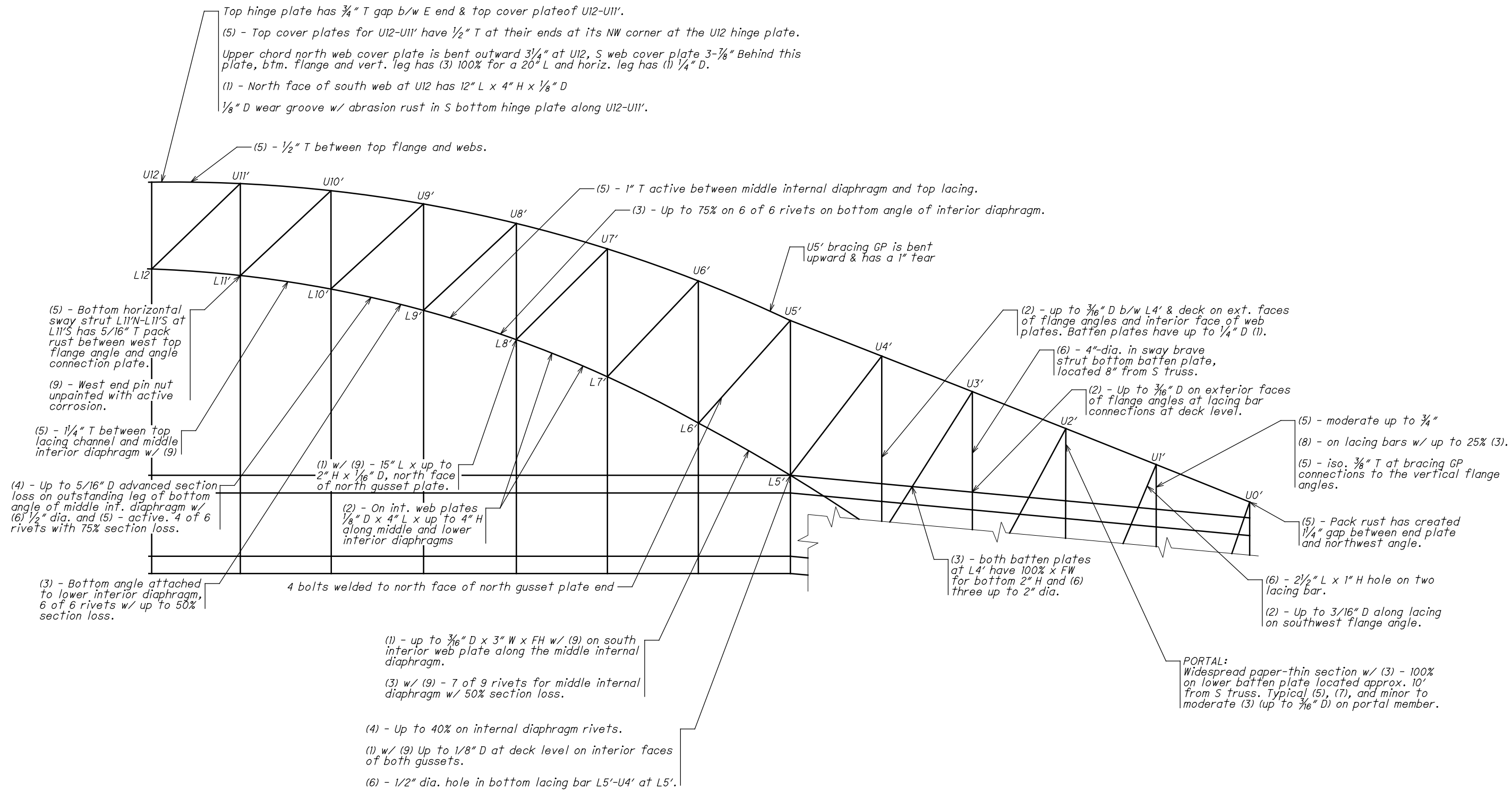
(1) - above L1-L2 has 30" L x 4" H x $\frac{3}{8}$ " D on S face (active, $\frac{1}{4}$ " remaining on original $\frac{3}{4}$ " T over this area)
 (1) - above L2-L3 has 30" L x 3" H x $\frac{1}{4}$ " D on S face 24" L x 2" H x $\frac{1}{8}$ " D on N face
 (6) - West edge stiffener has 1"-dia. and 4"-dia. holes in N conn. angle

- 1 - Pitting
- 2 - Painted Over Pitting
- 3 - Section Loss
- 4 - Painted Over Section Loss
- 5 - Pack Rust
- 6 - Corrosion Hole / Perforation
- 7 - Laminar Corrosion
- 8 - Surface Corrosion
- 9 - Active Corrosion

SOUTH TRUSS ELEVATION
 (BELOW DECK)

Uphill transverse angles have debris causing pitting and perforations of the diaphragm plates and transverse angles on lower chord between L0 To L5.

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SOUTH TRUSS ELEVATION
(ABOVE DECK)

- 1 - Pitting
- 2 - Painted Over Pitting
- 3 - Section Loss
- 4 - Painted Over Section Loss
- 5 - Pack Rust
- 6 - Corrosion Hole / Perforation
- 7 - Laminar Corrosion
- 8 - Surface Corrosion
- 9 - Active Corrosion

TYPICAL NOTES

1. BOTTOM 2 RIVETS AT DECK LEVEL FOR ALL WEST FLANGE ANGLES HAVE 75% - 90% SECTION LOSS.

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L3' GPs, BOTH:

(1) - 1/4" on ext faces along top of L3'L2' & L4'L3'. (1) - 1/8" x 2" H x 20" L on int. faces along L.C. at both ends

L3' IB GP:

(2) - 3/16" on N face along vertical.

(6) - W edge stiffening diaphragm & conn. angles have several up to 1-1/2"

L4 GPs, BOTH:

(1) - 5/16" x 3" H along top of L4'L3'
 (1) - 1/4" on all faces along top of L5'L4' (apx. 1/16" remaining for 10" L where chord is on both faces, 3/16" remaining where chord on outside face only)

L4 IB GP:

(1) - 1/8" x 2" H x 4" L on N face along top of L4'L3'.

L4 OB GP:

(1) - 1/16" x 2" H x 4" L on S face along top of L4'L3'.

(1) - 3/16" D x 3" L x 1/2 Height on int. web plates along int. diaphragms.

(2) - iso. 1/8" D on int. web plates and 3/16" on top flg. angle splice plates at L4'.

(8) - moderate
 (7) - 1/4" D x 2" L x FH interior web along internal diaphragm.

L2' GPs, BOTH:

(7) - active along lower chords.
 (1) - 1/8" x 3" H on both faces along L2'L1'.

L2' IB GP:

(1) - 1/4" x 4" H x 30" L on N face along L3'L2'.
 (2) - 1/8" x 3" H x 20" L on S face along L3'L2'.

L2' OB GP:

(1) - 3/8" x 3" H x 10" L on N face along L2'L1'.
 (1) - 1/8" x 2" H x 20" L on N face along L3'L2'.
 (1) - isolated 1/8" on OB face along FL

(3) - up to 100% on lower L1' batten plate along lower 18"

(3) - Lower 2 lacing bars have 75% at the conn. to the flgs. and one has a 2" dia. (6) on W face.

(1) - 1/8" (Iso. 3/16") D x FW on flg. angles along lacing.

L1' GPs, BOTH:

(9) w/ (1) - up to 3/16" x 2" H along strut connections and along lower chord on outside faces over FL.

(1) - 3/16" x 4" H x 16" L on inside faces along L1'-L0'.

(1) - active 3/16" x 3" H x 20" L on inside faces along L2'L1'

(6) - Two 2" dia. holes in west edge stiff. diaph.

(8) - moderate w/ iso. (7) & (6) t/o the L.C. diaphragm plates.

(1) - 3/16" D x FW on top face of N top flg. within limits of L0' GP plus 3/16" D x 3" H on the vertical leg w/ 100% rivet head loss.

(2) - int. web plates have 1/4" D reactivating x Fh x 2" L along internal diaphs.

(6) - Int. diaph. at L1' has FW x 4" H hole along btm. angle

(6) - Btm. lacing channels have iso. holes up to 2" dia.

(6) - 2"-dia. in top lacing at middle diaphragm.

(3) - advanced, vert truss to lower FB knee braces. Btms. of connection angles have 100% x 8" H.

(6) - Bottom batten plate has two 4"-dia. holes, lacing bar has one 1/2"-dia. hole.

(8) - moderate with widespread peeling paint to components below the lower deck w/ (5) up to 3/8" T b/w lacing bars.

Numerous replaced lacing bars at the lower deck connection to the vertical.

(2) - t/o lower FB connection and missing batten plates.

(7) - int. web plates up to 1/4" T

(6) - 4" x 1" Cantilever brace connection, west face.

(3) - 100% on lower batten plate along the lower 12" & additional 6" x 4" hole. Upper batten plate is almost 100% gone w/ five lacing bars with (6) 2-4" in dia.

(8) - moderate w/ iso. (6) up to 5" dia. noted t/o the L.C. diaphragm plates.

(1) - 1/4" D x 3" L x 3/4 Height on int. web plates along int. diaph.

Isolated lacing channels replaced.

(6) - 3"-dia. in L2'-L1' top batten plate at L1'.

(7) w/ (1) - up to 1/8" D x 5" H along bottom of L1' N web splice.

Paint peeling w/ iso. (9) over Full Length.

(3) - iso. areas of advanced (3/16" D x 4" W) on int. web plates around the rivets & adj. to lacing.

(5) - 1/4" b/w angles & web plates

(1) - up to 3/16" D on flg. angles along lacing.

Lacing bars deformed due to pack rust up to 3/4" T b/w lacing & flanges

Heavy debris in panels at strut.

Non-structural BRG pin cover plates: 6-1/2" L crack on both faces.

Moderate debris build up t/o BRG thrust block.

(5) - active 1" T b/w L.C. & BRG full perimeter of the pin

(2) - up to 3/16" D on BRG casting around pin nut.

(3) - iso. moderate to advanced on BRG casting w/ some areas of (7 & 9)

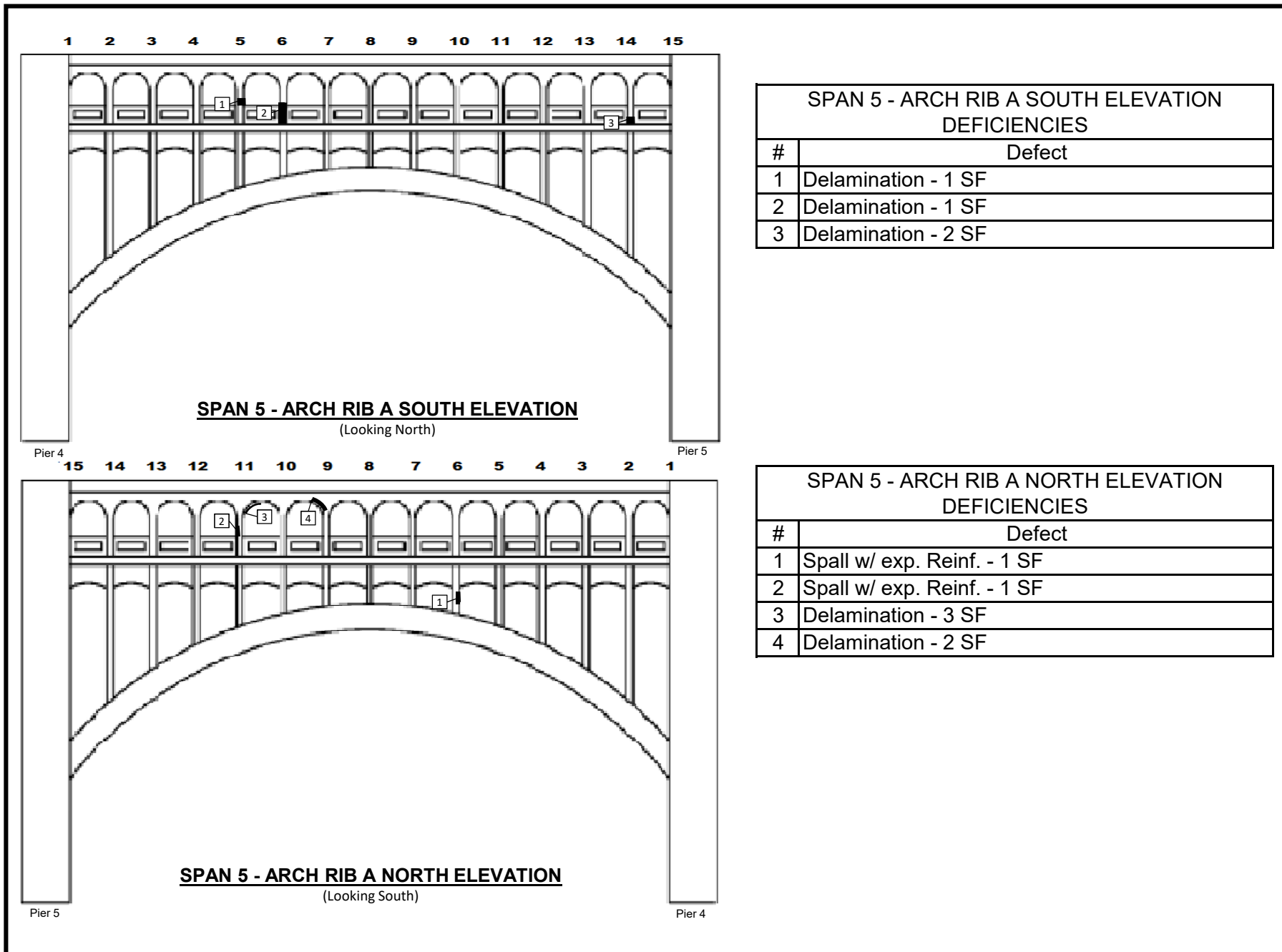
L0' GPs, BOTH:
 (7) at base of interior.

L0' N GP:
 (1) - 3/16" D x 5" H x 41" L along lower chord.

- 1 - Pitting
- 2 - Painted Over Pitting
- 3 - Section Loss
- 4 - Painted Over Section Loss
- 5 - Pack Rust
- 6 - Corrosion Hole / Perforation
- 7 - Laminar Corrosion
- 8 - Surface Corrosion
- 9 - Active Corrosion

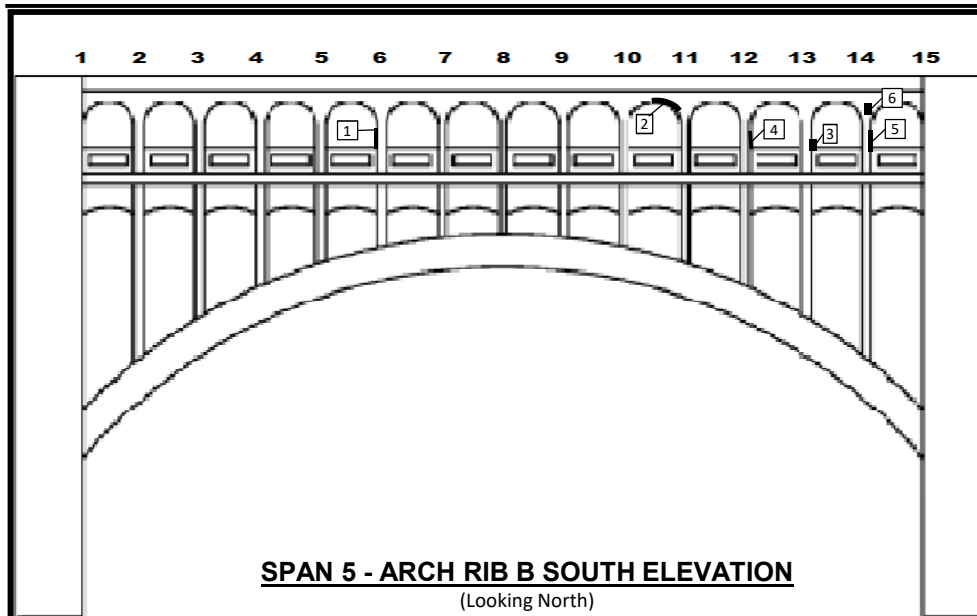
SOUTH TRUSS ELEVATION
 (BELOW DECK)

NOT TO SCALE		DETROIT-SUPERIOR BRIDGE	
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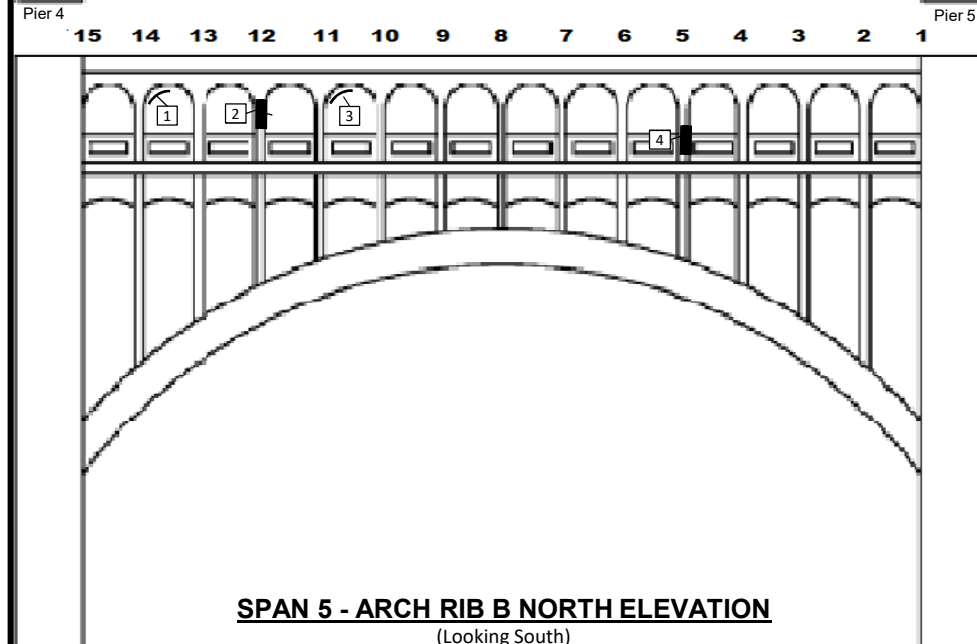
SPAN 5 - ARCH RIB A SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 1 SF
2	Delamination - 1 SF
3	Delamination - 2 SF

SPAN 5 - ARCH RIB A NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 1 SF
2	Spall w/ exp. Reinf. - 1 SF
3	Delamination - 3 SF
4	Delamination - 2 SF



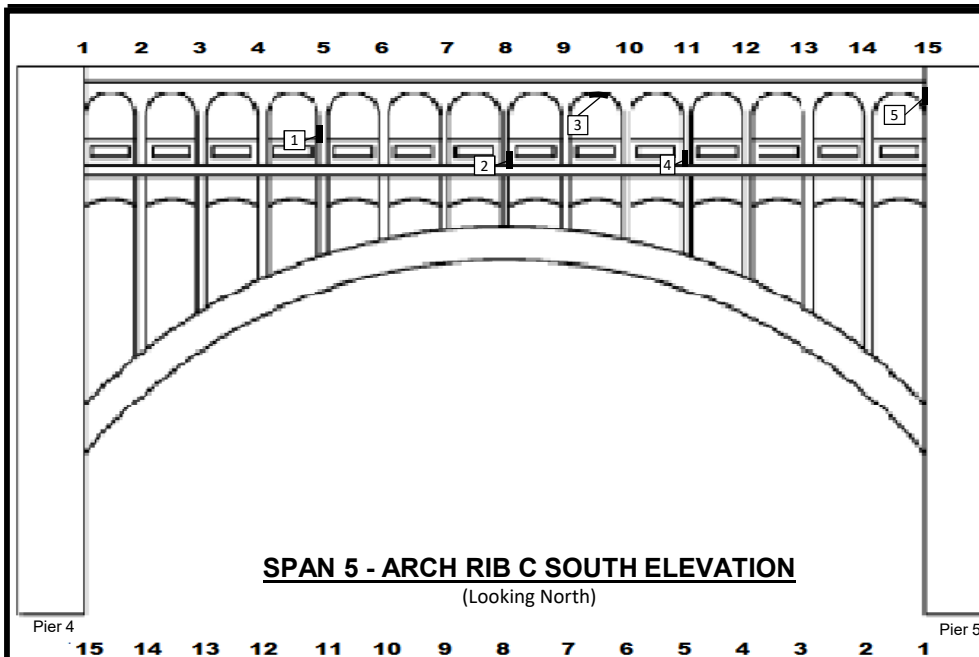
SPAN 5 - ARCH RIB B SOUTH ELEVATION
(Looking North)

SPAN 5 - ARCH RIB B SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall - 1 SF, 3" D
2	Delamination - 2 SF
3	Delamination - 1 SF
4	Delamination - 2 SF
5	Spall w/ exp. Reinf - 3 SF, 2" D
6	Delamination - 2 SF

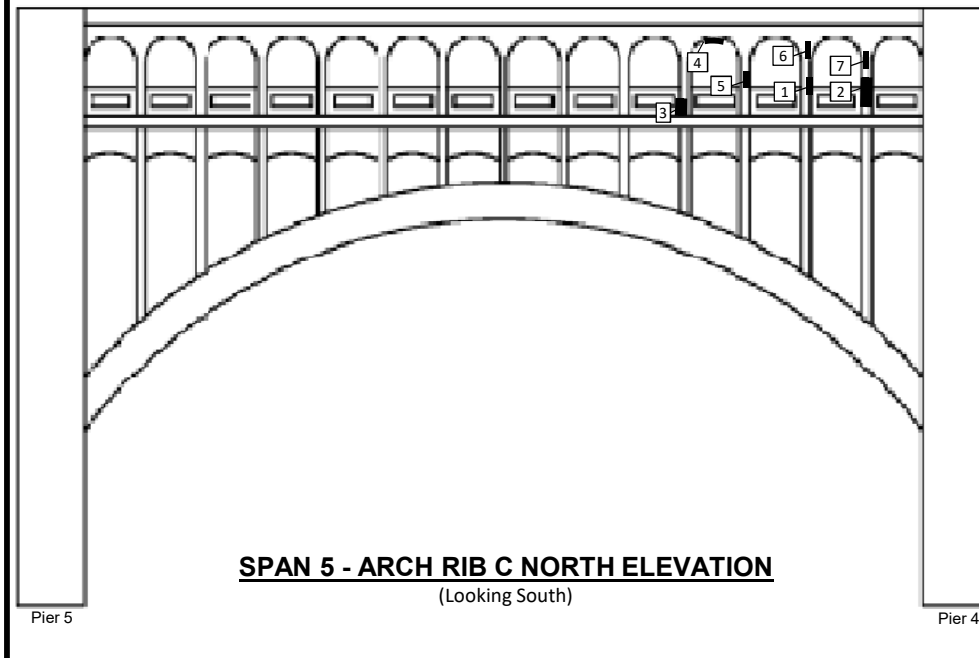


SPAN 5 - ARCH RIB B NORTH ELEVATION
(Looking South)

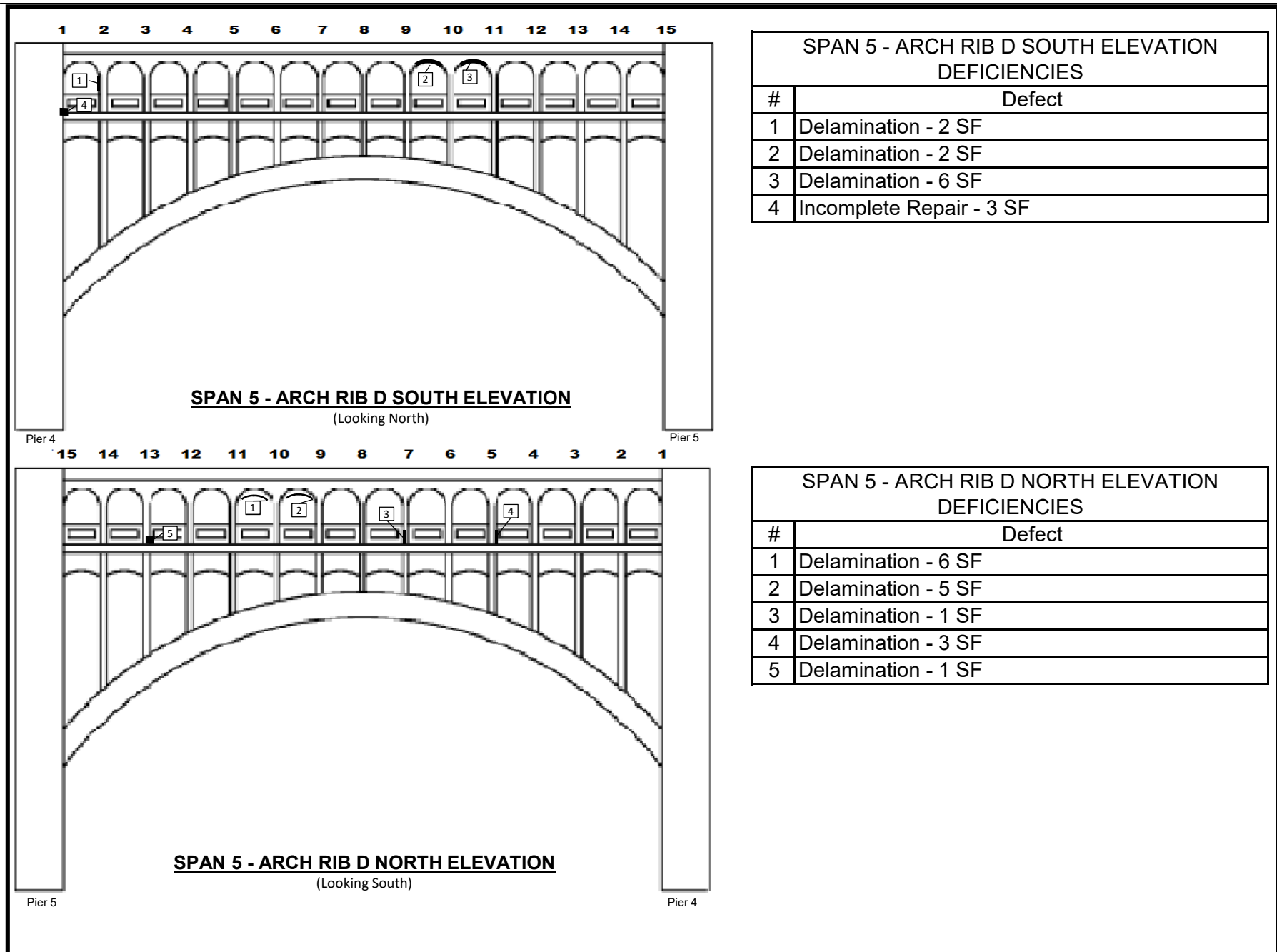
SPAN 5 - ARCH RIB B NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 3 SF
2	Spall w/ exp. Reinf. - 4 SF
3	Spall w/ exp. Reinf. - 2 SF
4	Delamination - 3 SF

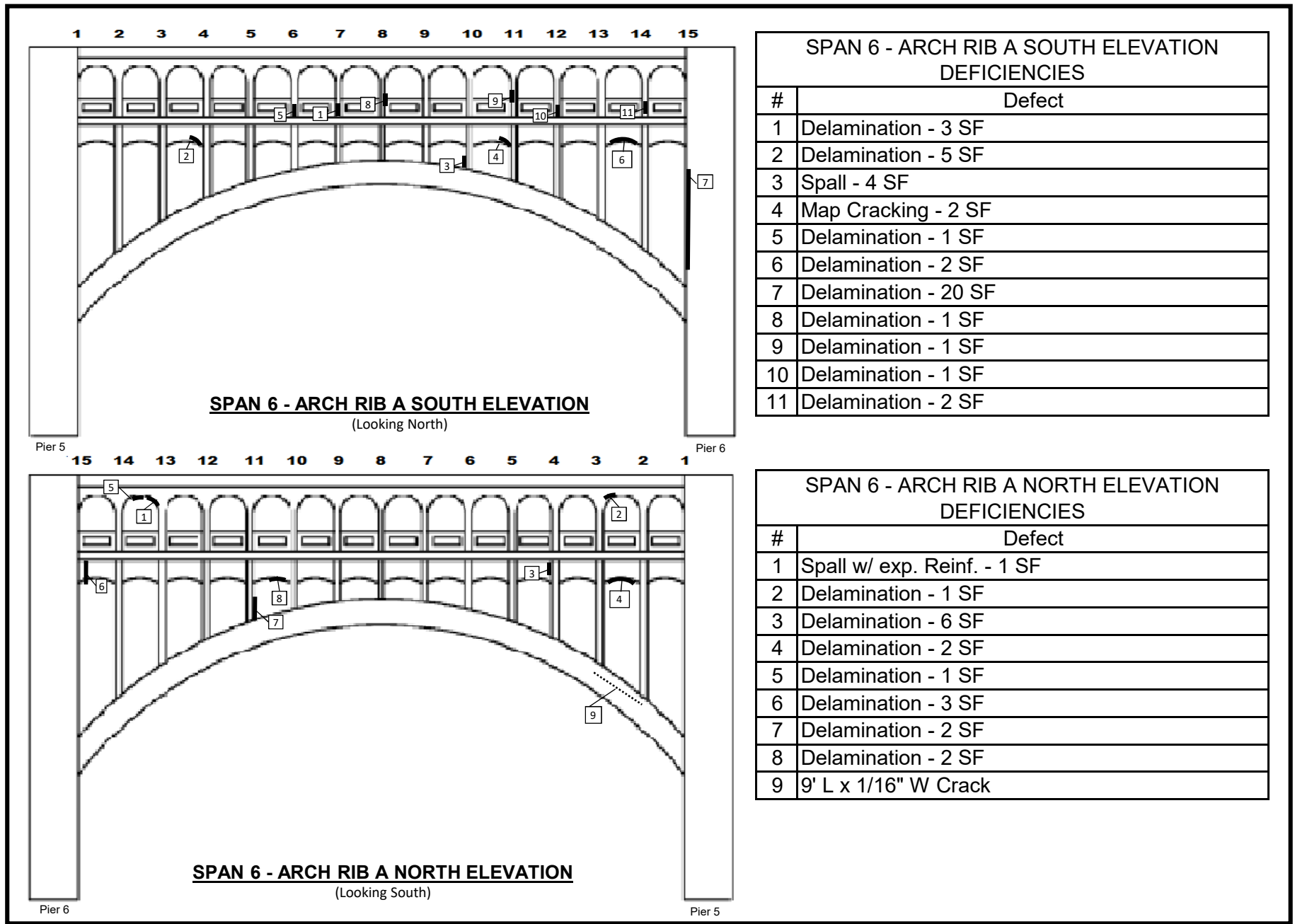


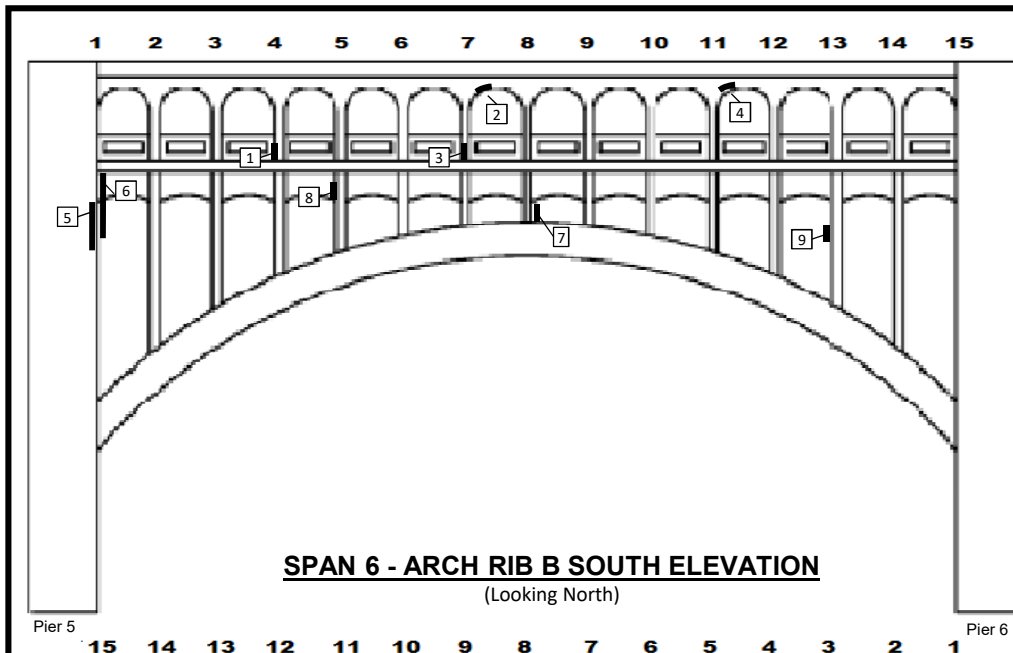
SPAN 5 - ARCH RIB C SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 1 SF
2	Delamination w/ Spall w/ exp. Reinf. - 4 SF
3	Spall w/ exp. Reinf. - 1 SF
4	Delamination - 4 SF
5	Spall w/ exp. Reinf. - 2 SF



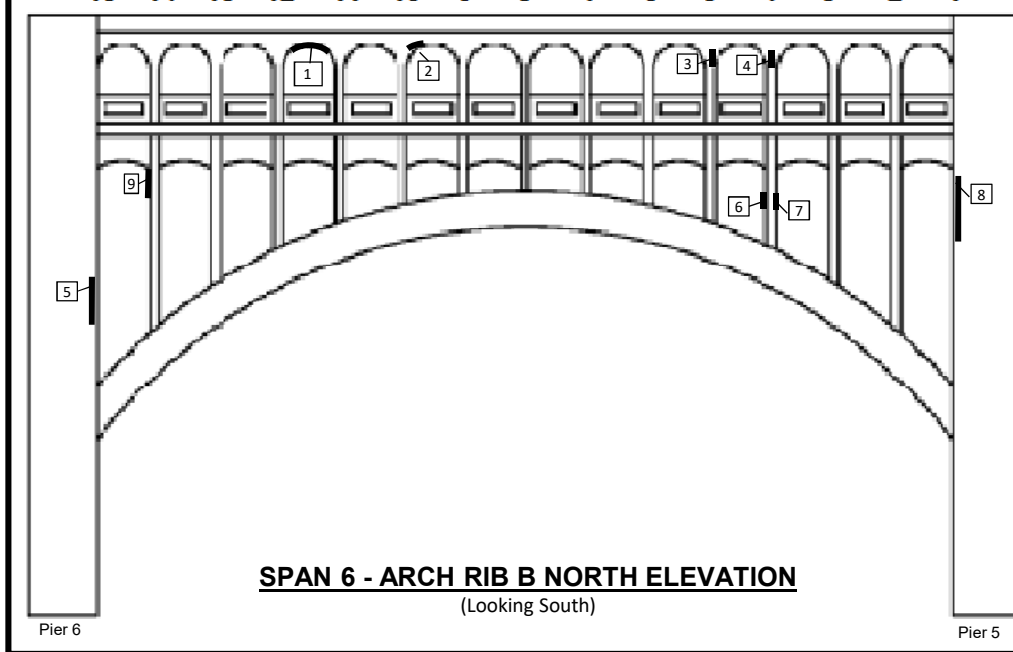
SPAN 5 - ARCH RIB C NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 1 SF, 1" D
2	Spall w/ exp. Reinf. - 1 SF, 1.5" D
3	Delamination - 2 SF
4	Delamination - 2 SF
5	Delamination - 2 SF
6	Delamination - 3 SF
7	Delamination - 5 SF



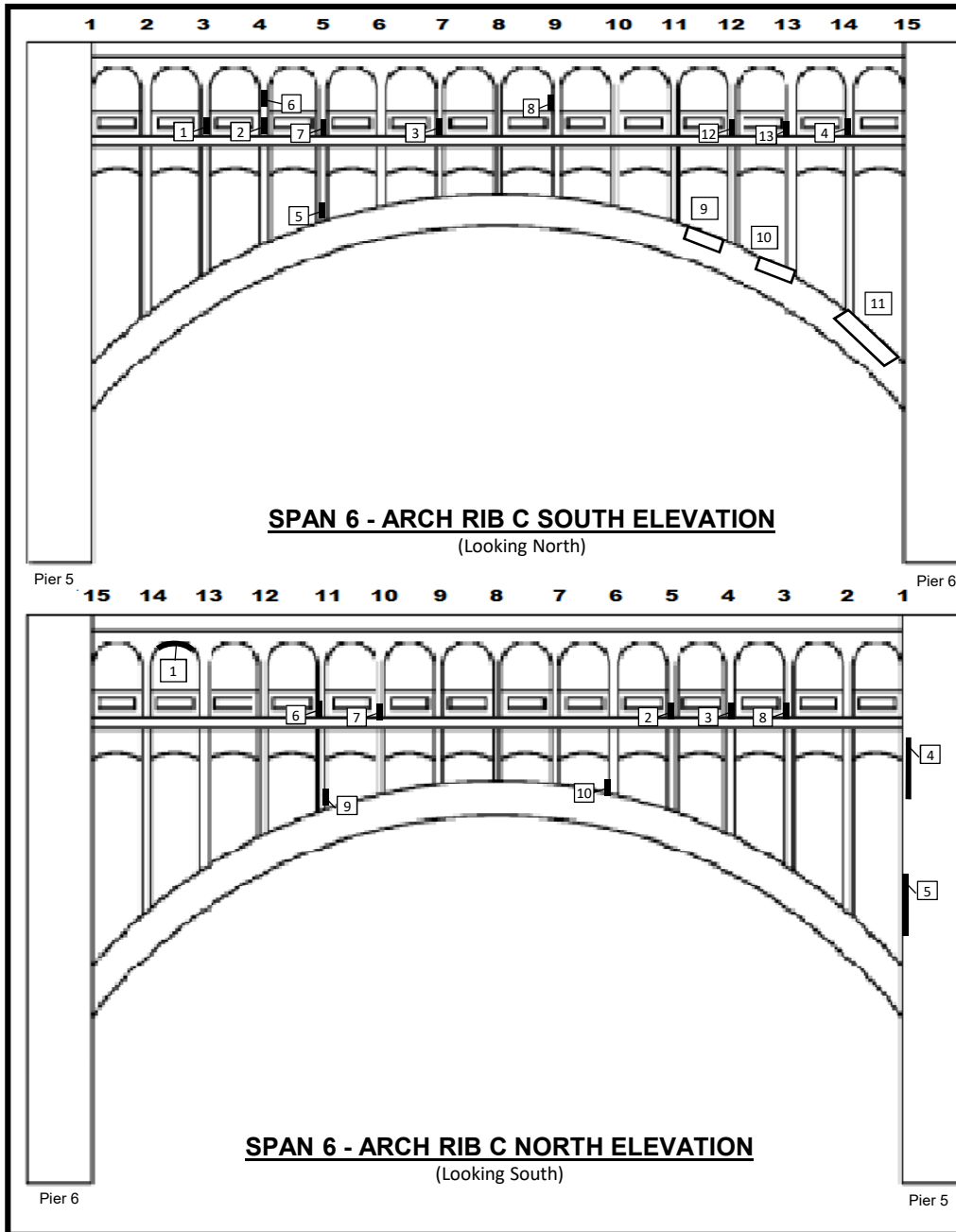




SPAN 6 - ARCH RIB B SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 2 SF
2	Delamination - 4 SF
3	Delamination - 3 SF
4	Spall - 4 SF, 4" D
5	Delamination - 16 SF
6	Delamination - 5 SF
	Spall w/ exp. Reinf. - 15 SF
7	Delamination - 1 SF
8	Delamination - 4 SF
9	Spall w/ exp. Reinf. - 16 SF

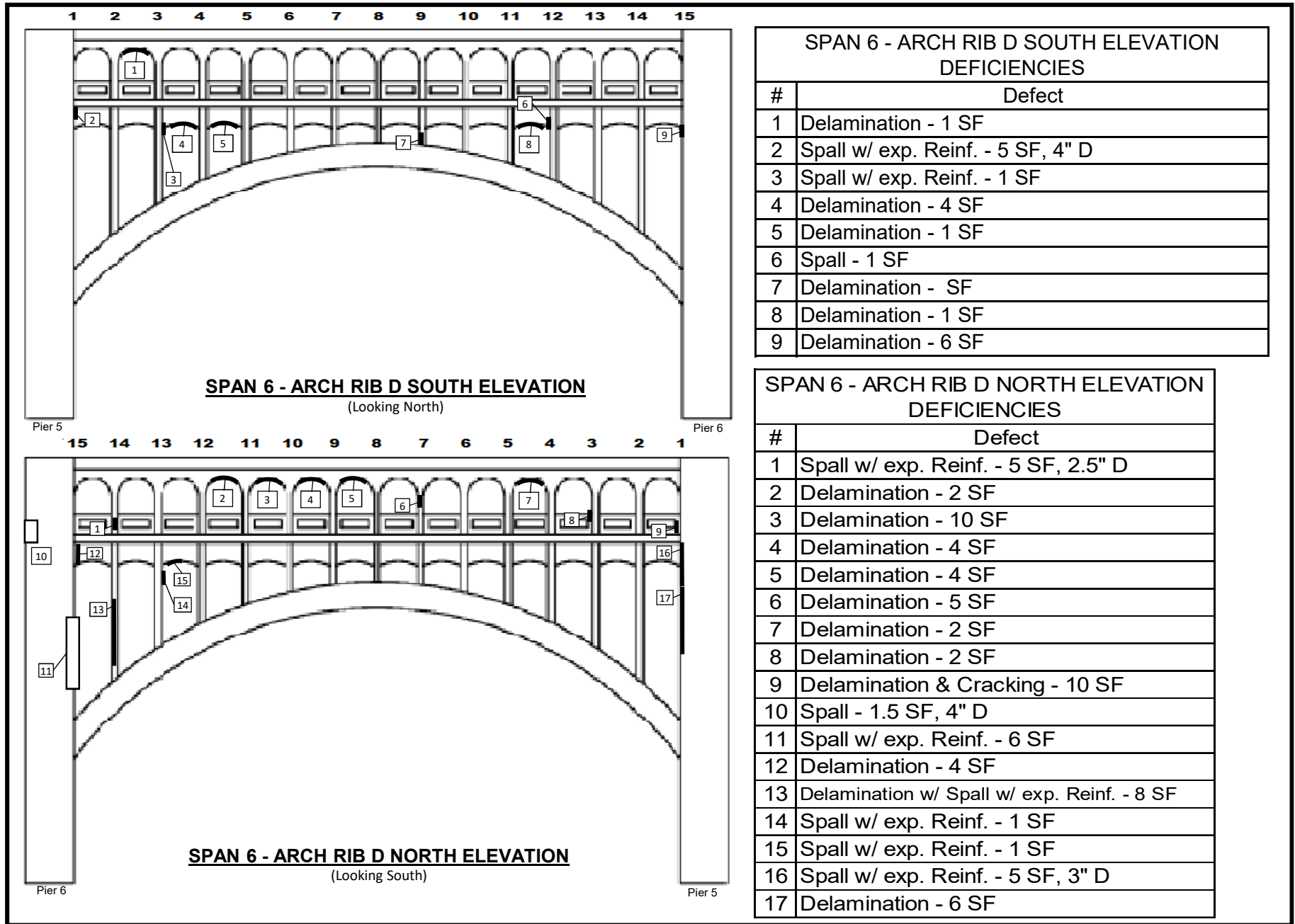


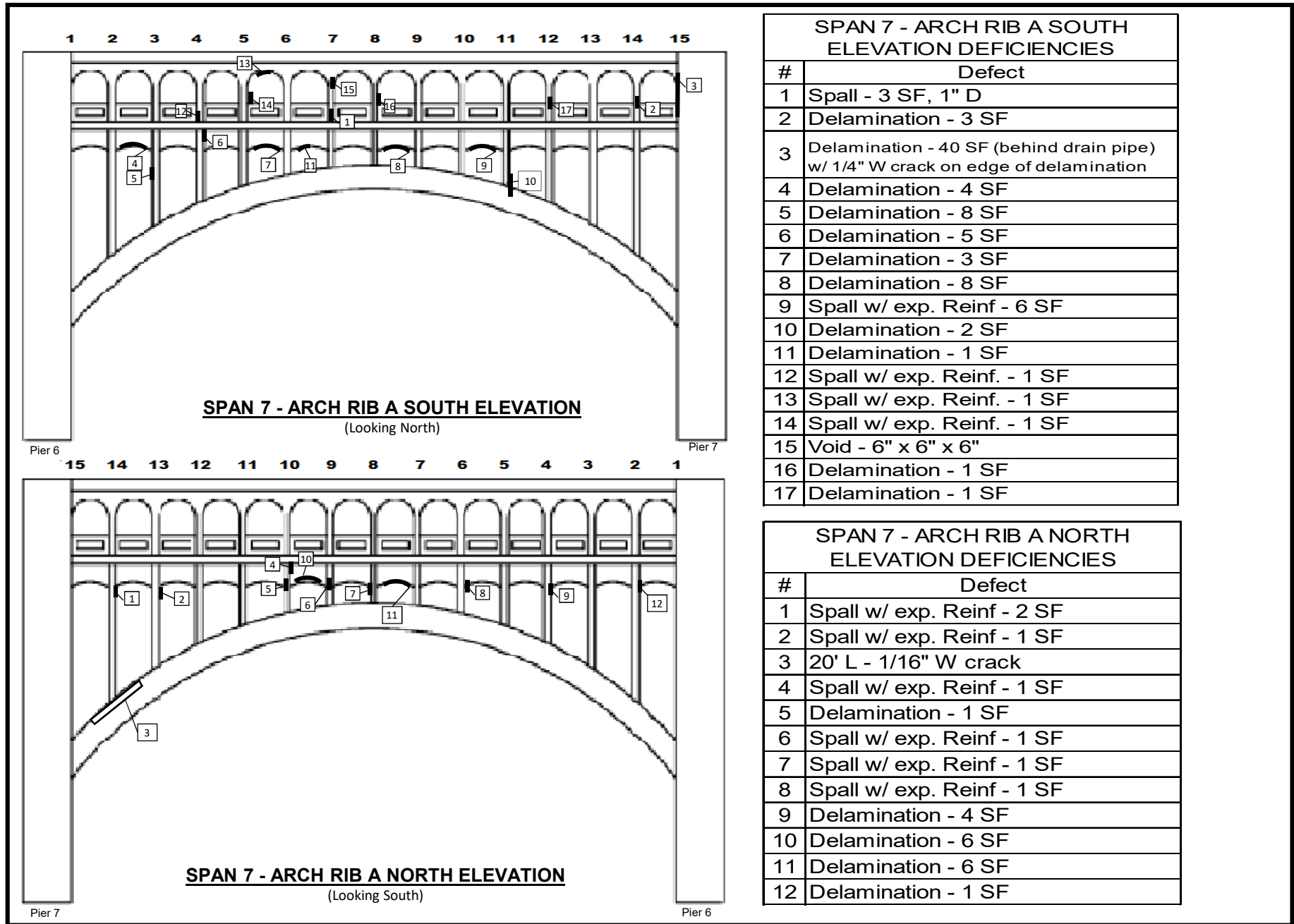
SPAN 6 - ARCH RIB B NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 24 SF
2	Spall w/ exp. Reinf. - 1 SF
3	Delamination - 4.5 SF
4	Delamination - 2 SF
5	Delamination - 8 SF
6	Delamination - 1 SF
7	Delamination - 1 SF
8	Delamination - 8 SF
9	Spall w/ exp. Reinf. - 2 SF

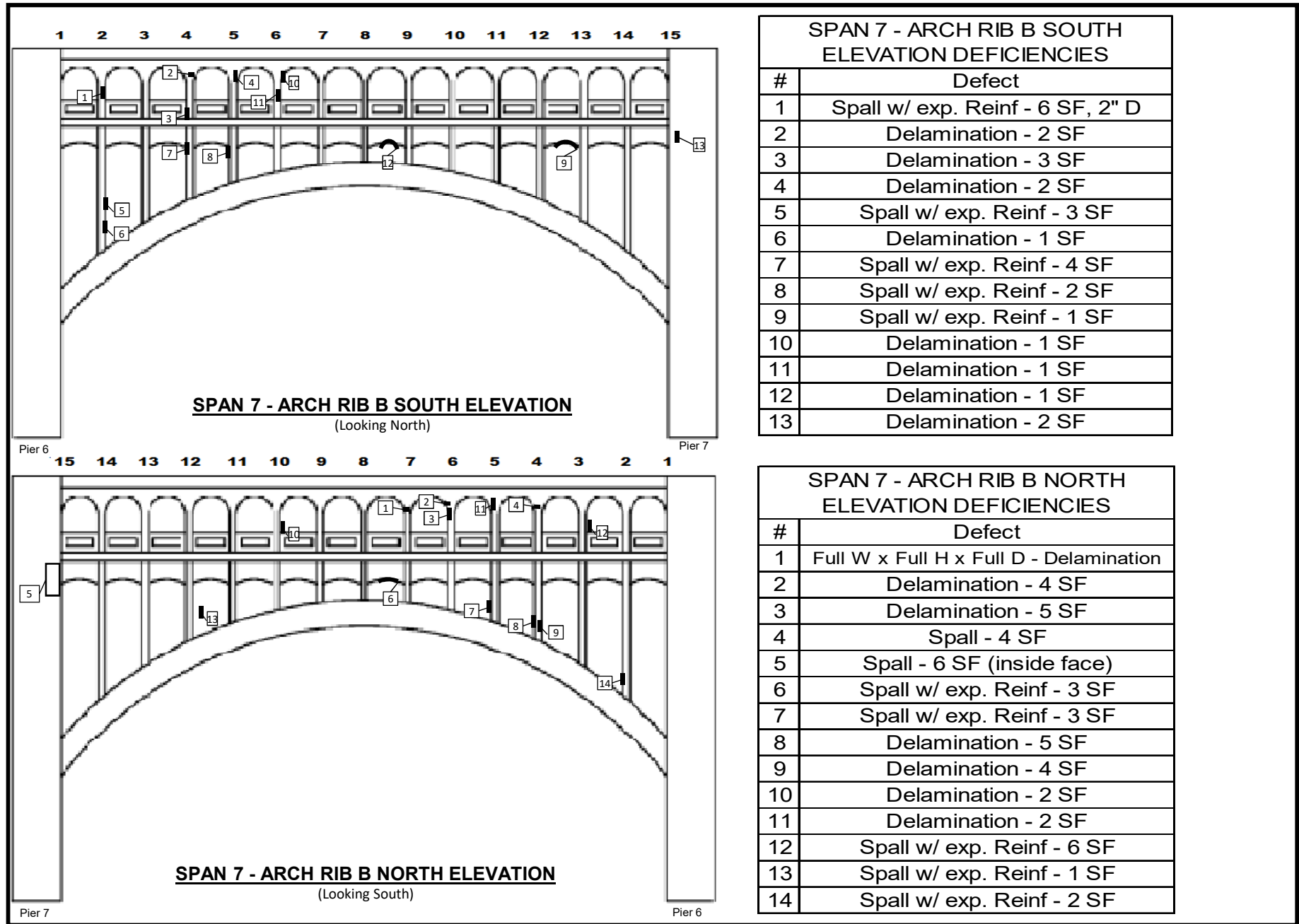


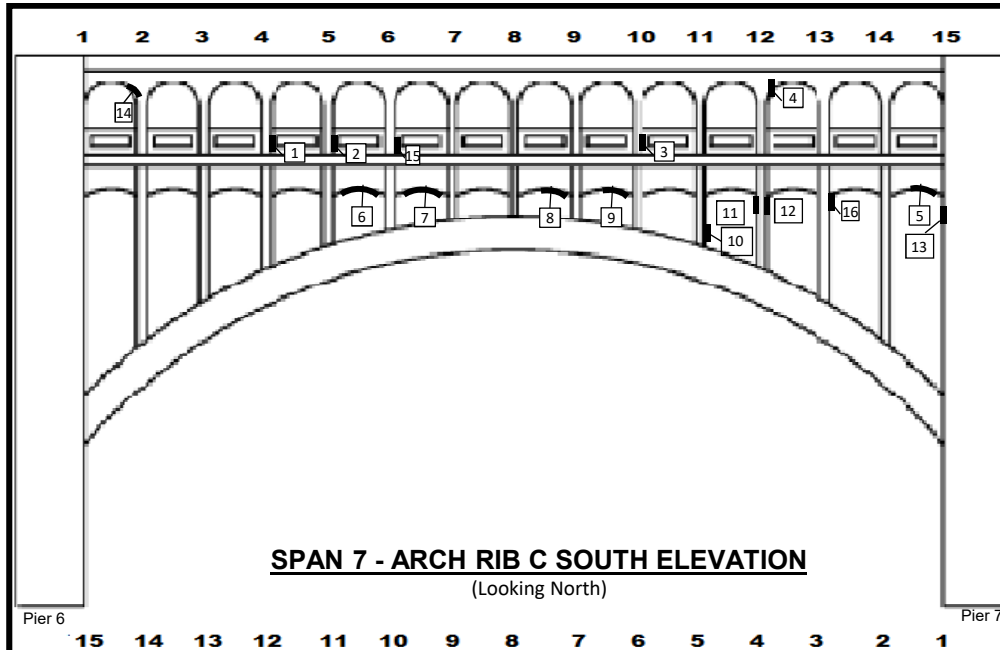
SPAN 6 - ARCH RIB C SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 2 SF, 2" D
2	Spall w/ exp. Reinf. - 1 SF, 3" D Delamination - 9 SF
3	Spall w/ exp. Reinf. - 1 SF, 2" D
4	Delamination - 3 SF
5	Spall w/ exp. Reinf. - 1 SF
6	Delamination - 1 SF
7	Delamination - 4 SF
8	Delamination - 1 SF
9	Delamination - 12 SF
10	Delamination - 10 SF Delamination - SF (topside of arch)
11	Delamination - 20 SF
12	Delamination - 2 SF
13	Delamination - 1 SF

SPAN 6 - ARCH RIB C NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 2 SF
2	Spall w/ exp. Reinf. - 1 SF
3	Delamination - 4 SF
4	Spall w/ exp. Reinf. - 10 SF
5	Spall w/ exp. Reinf. - 15 SF
6	Delamination w/ Spall w/ exp. Reinf - 3 SF
7	Delamination - 3 SF
8	Delamination - 4 SF
9	Delamination - 1 SF



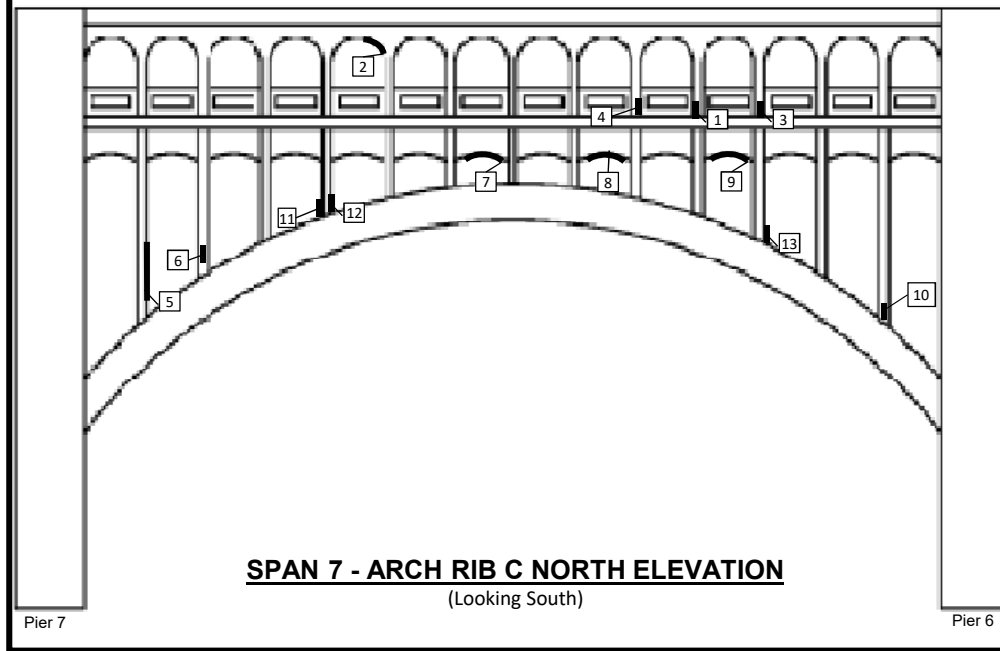






SPAN 7 - ARCH RIB C SOUTH ELEVATION

(Looking North)

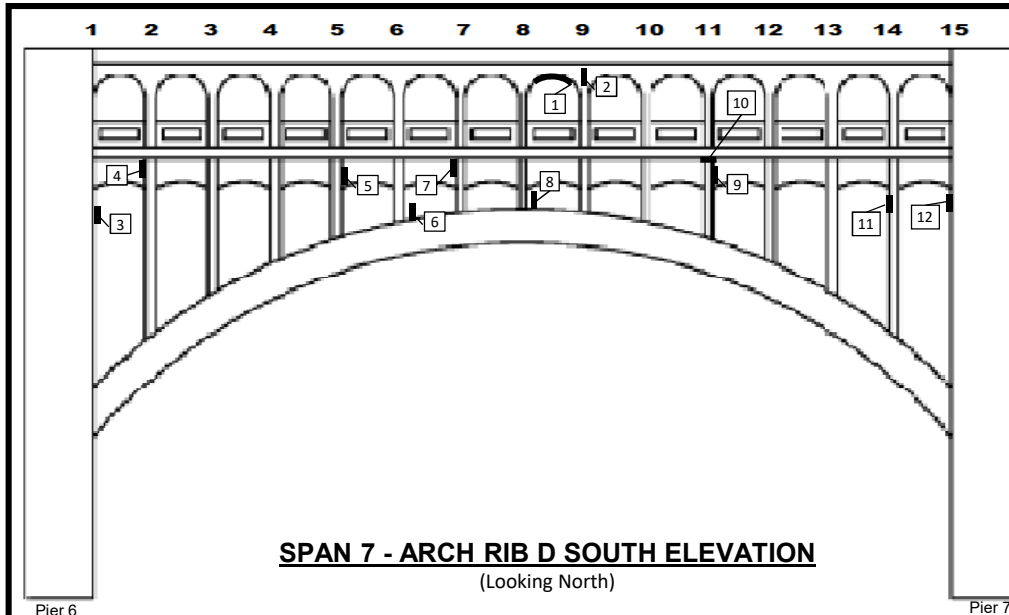


SPAN 7 - ARCH RIB C NORTH ELEVATION

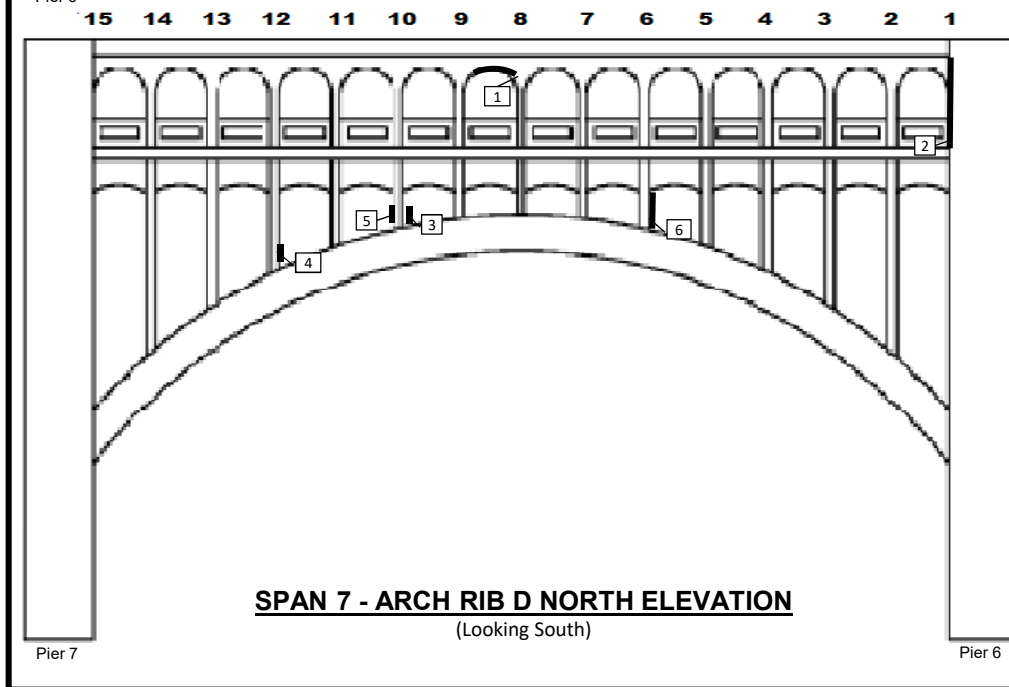
(Looking South)

SPAN 7 - ARCH RIB C SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 3 SF
2	Delamination w/ Spall w/ exp. Reinf. - 5 SF, 3" D
3	Delamination - 2 SF
4	Spall w/ exp. Reinf - 2 SF, 2" D
5	Delamination - 6 SF
6	Delamination - 6 SF
7	Delamination - 4 SF
8	Delamination - 4 SF
9	Delamination - 4 SF
10	Delamination - 1 SF
11	Delamination - 4 SF
12	Spall w/ exp. Reinf. - 1 SF
13	Delamination - 6 SF
14	Delamination - 2 SF
15	Delamination - 2 SF
16	Delamination - 1 SF

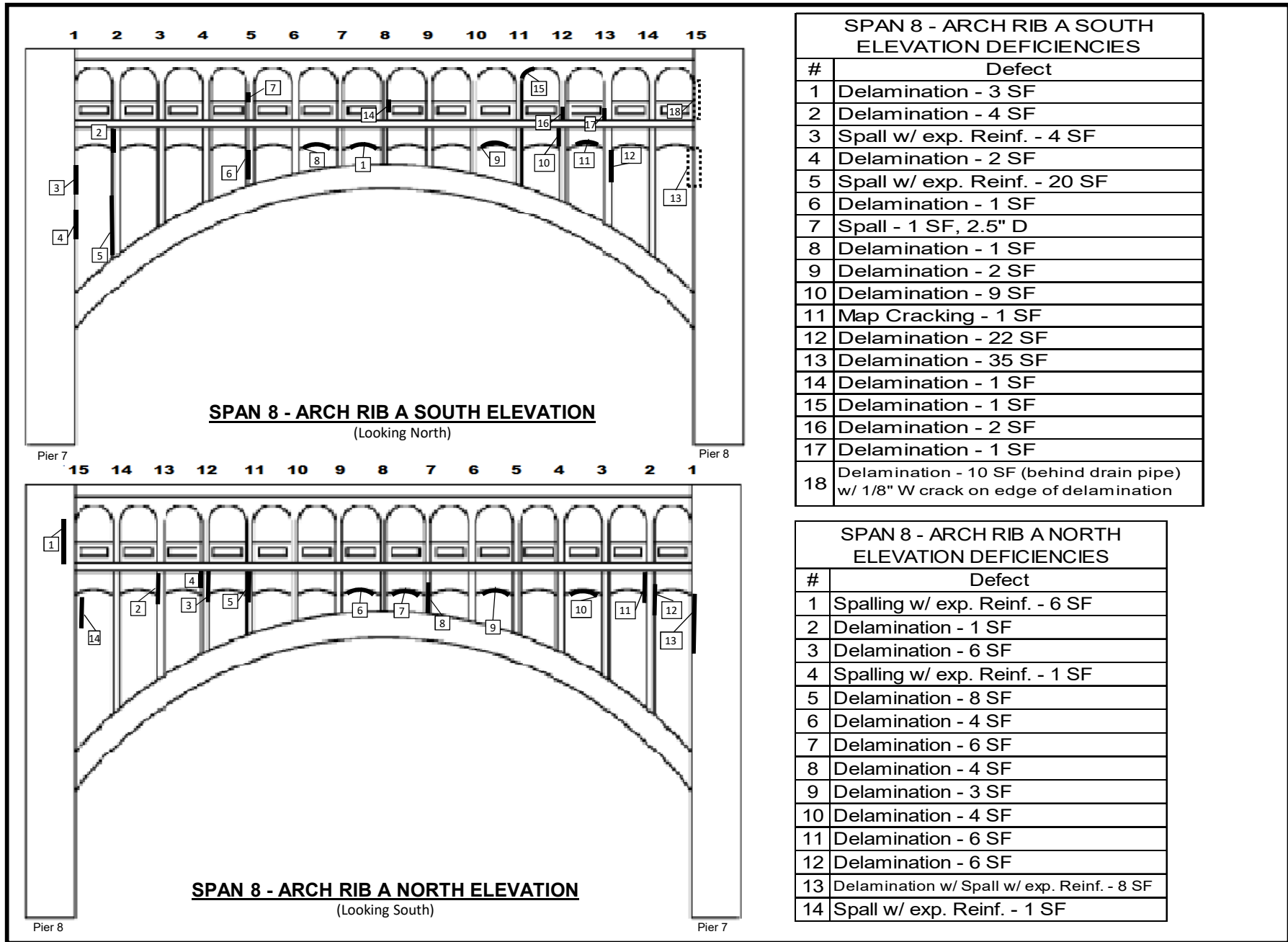
SPAN 7 - ARCH RIB C NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 2 SF
2	Spall w/ exp. Reinf - 4 SF, 3" D
3	Delamination - 2 SF
4	Delamination w/ Spall w/ exp. Reinf - 2 SF, 1.5" D
5	Delamination - 20 SF
6	Delamination - 2 SF
7	Delamination - 10 SF
8	Delamination - 2 SF
9	Delamination - 3 SF
10	Delamination - 3 SF
11	Delamination - 1 SF
12	Delamination - 1 SF
13	Spall w/ exp. Reinf. - 1 SF

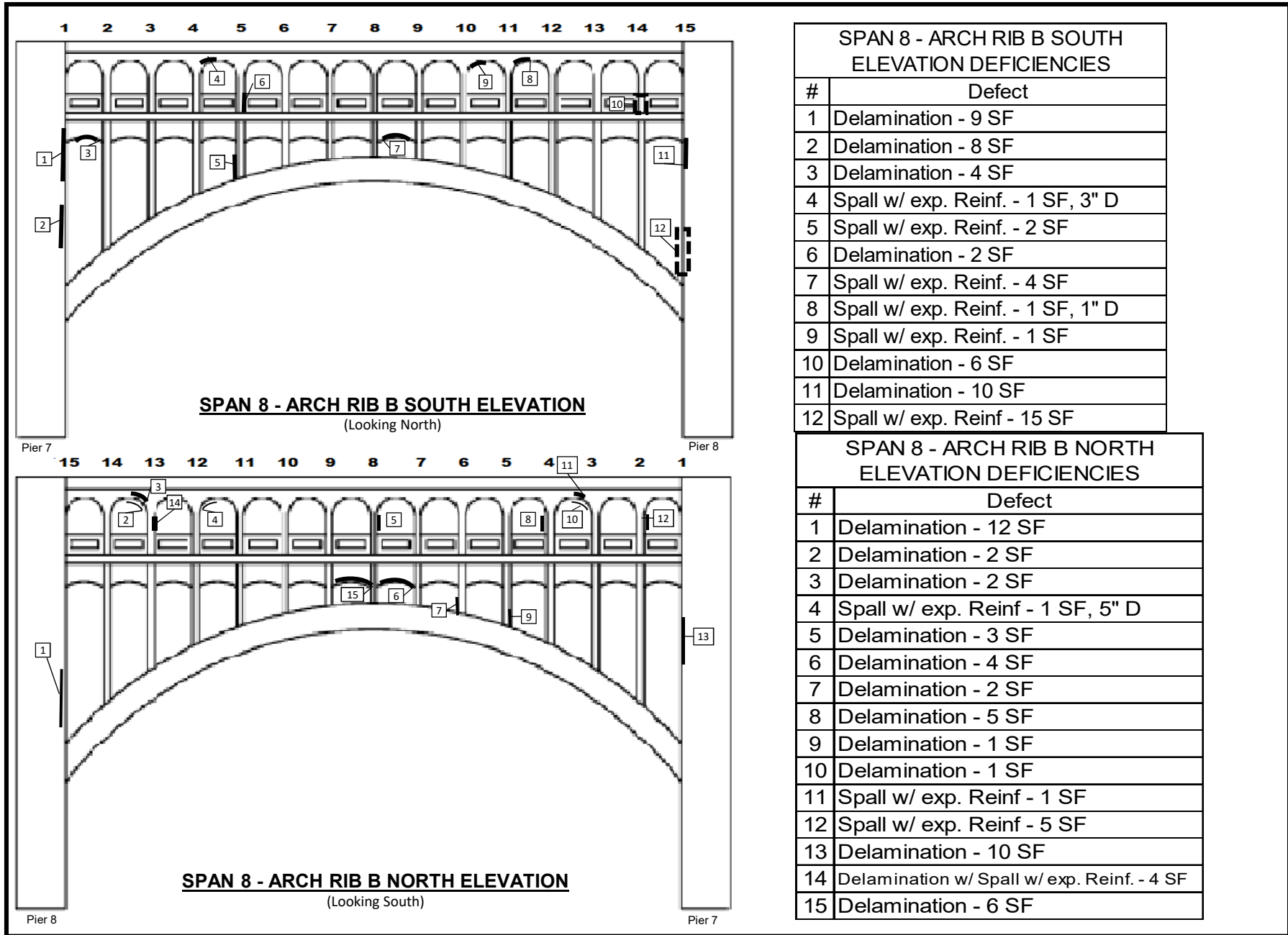


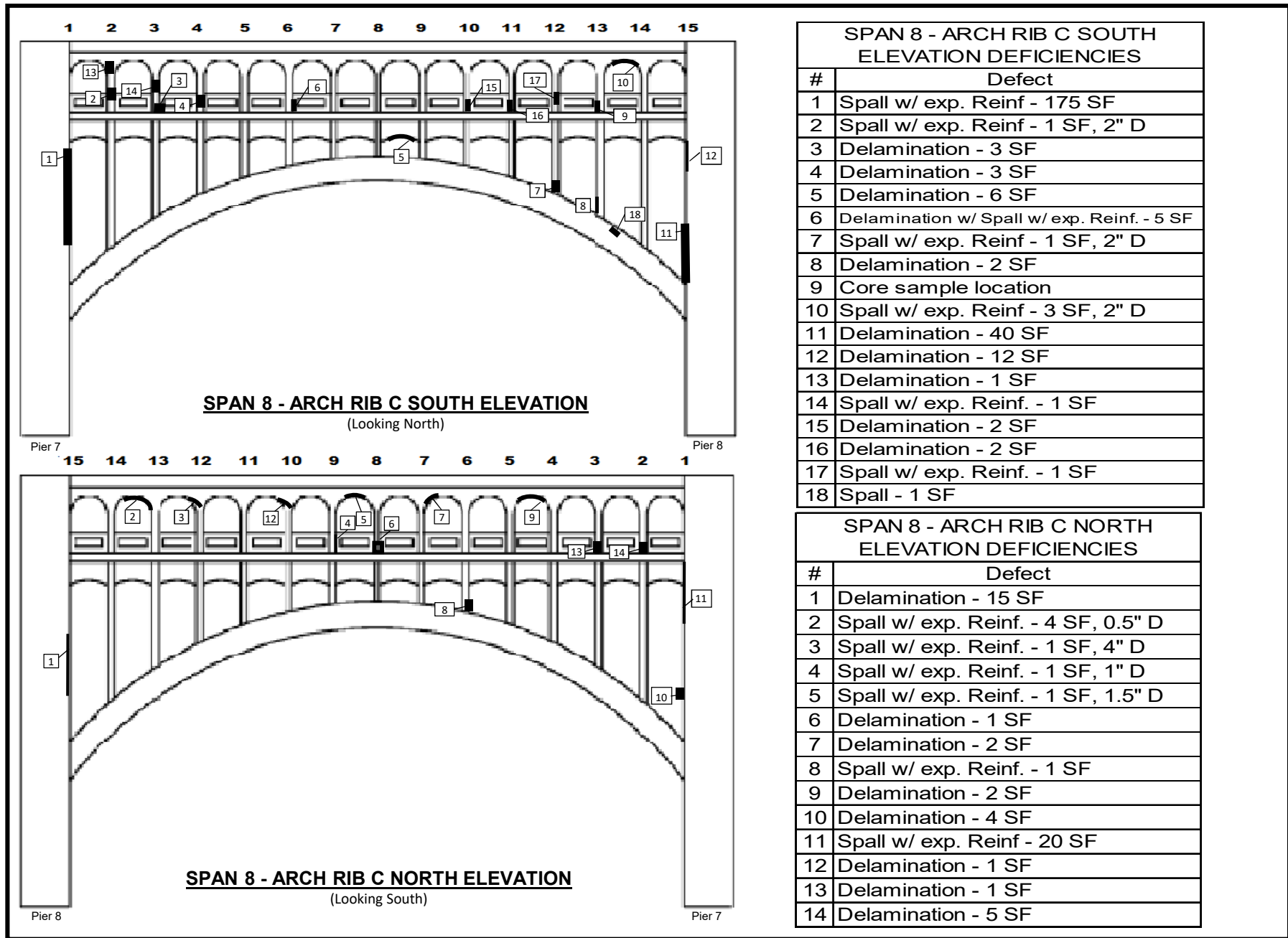
SPAN 7 - ARCH RIB D SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 4 SF
2	Delamination - 5 SF
3	Delamination - 4 SF
4	Delamination w/ Spall w/ exp. Reinf. - 12 SF
5	Delamination - 3 SF
6	Delamination - 2 SF
7	Delamination - 3 SF
8	Spall w/ exp. Reinf - 1 SF
9	Delamination - 3 SF
10	Delamination - 6 SF
11	Delamination - 6 SF
12	Delamination - 4 SF

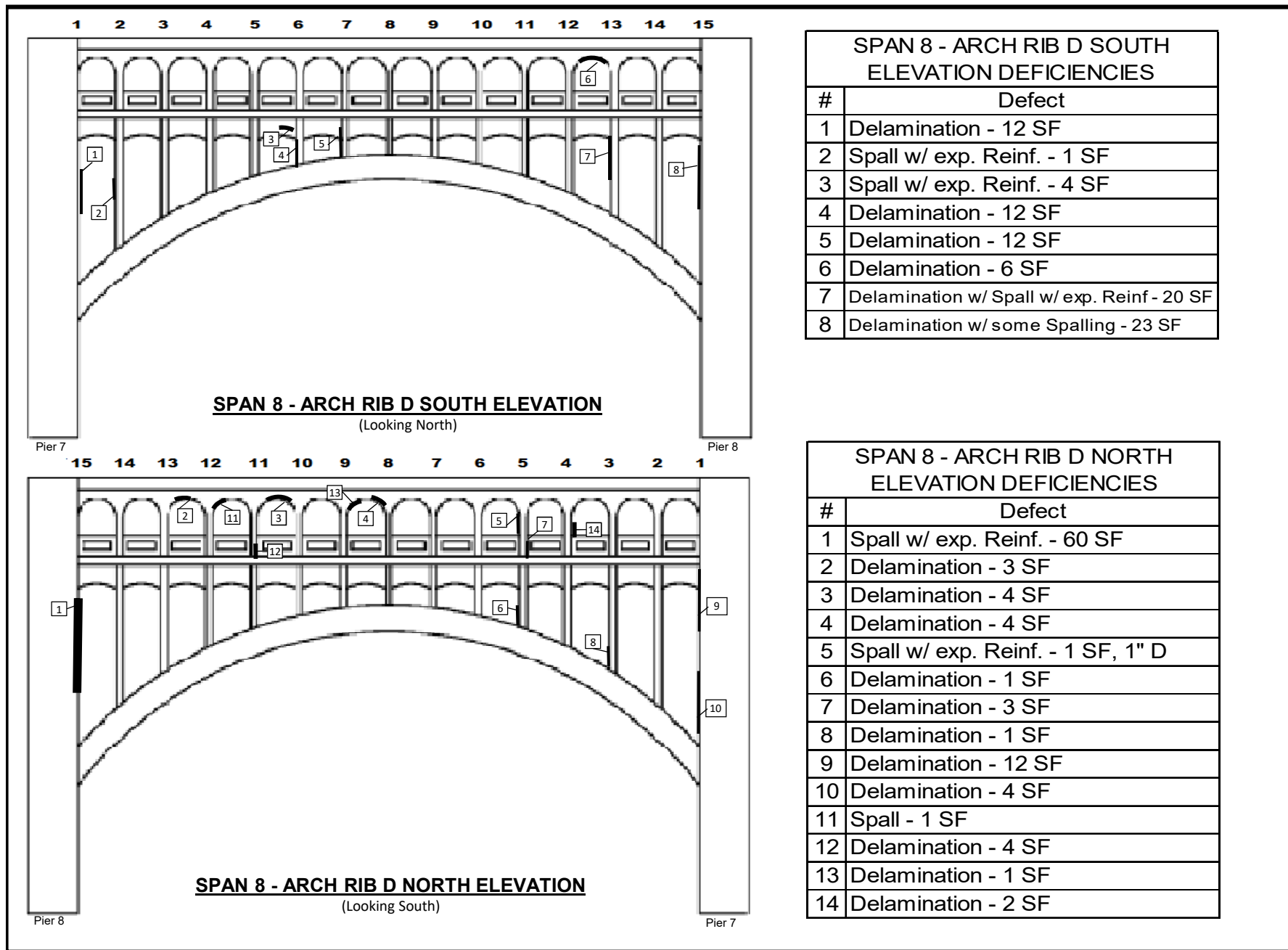


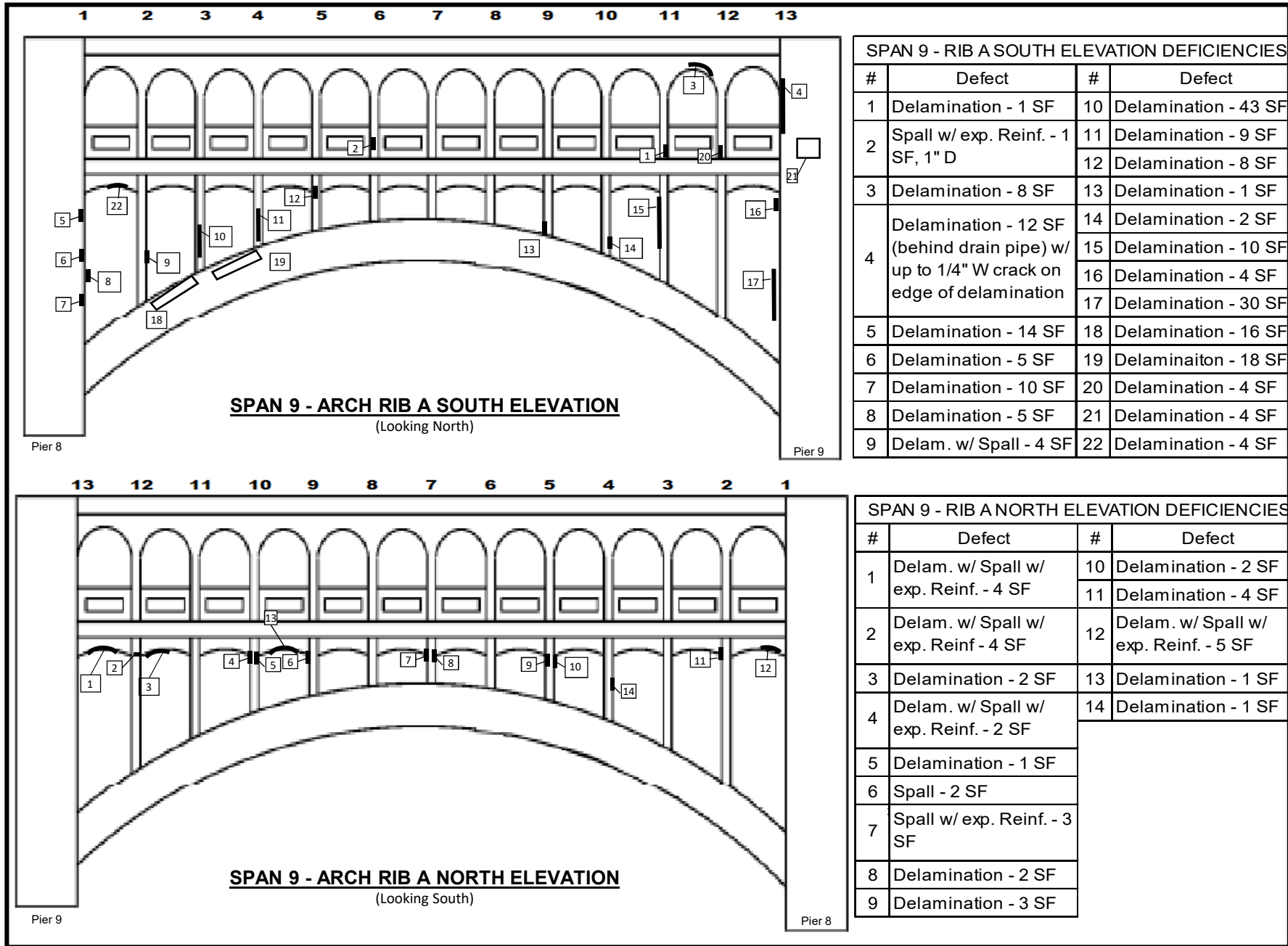
SPAN 7 - ARCH RIB D NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 4 SF
2	5' Tall x Full Depth x 1/8" W crack
3	Spall w/ exp/ Reinf. - 1 SF
4	Delamination - 1 SF
5	Spall w/ exp. Reinf. - 1 SF
6	Delamination - 6 SF

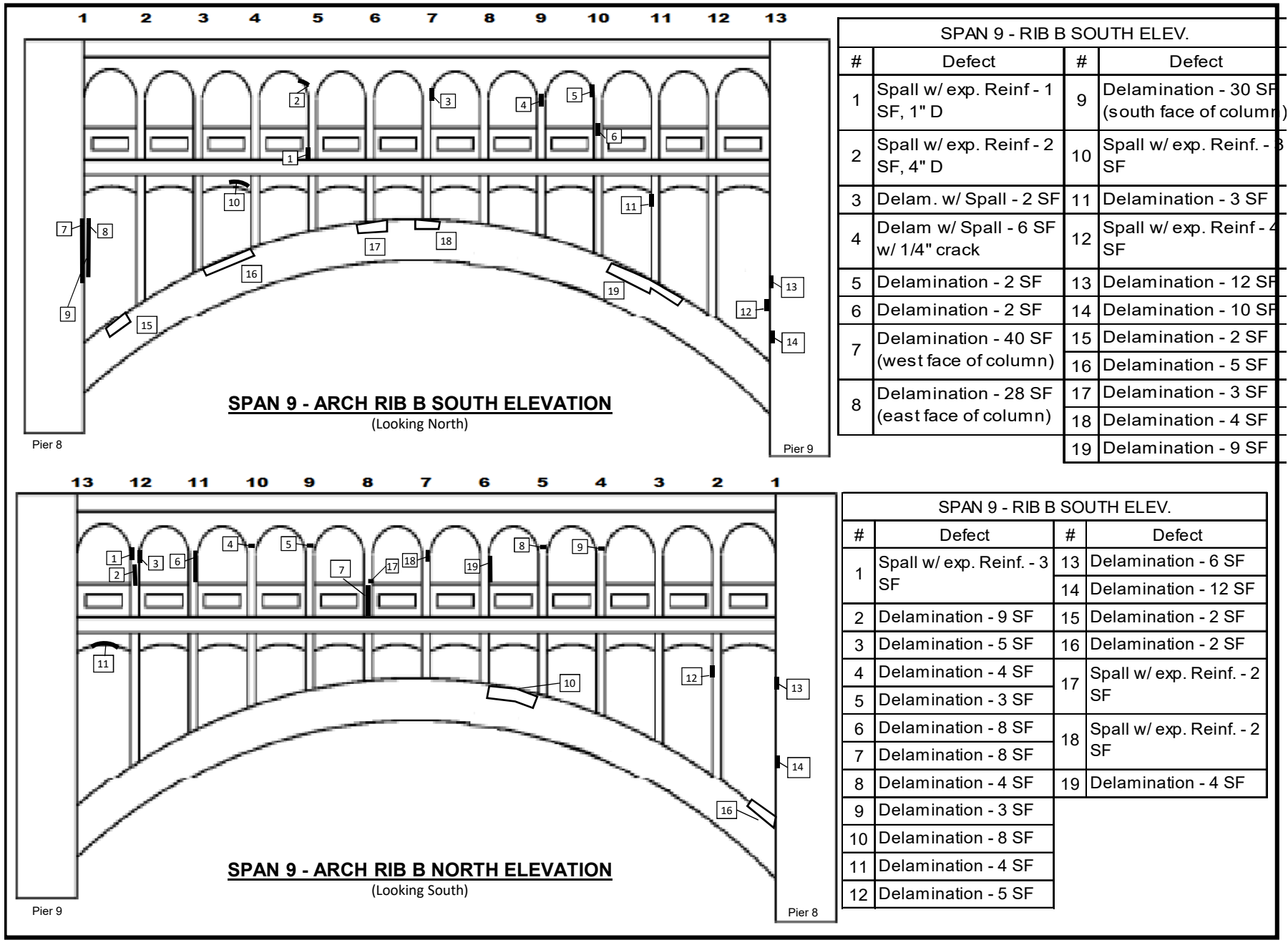


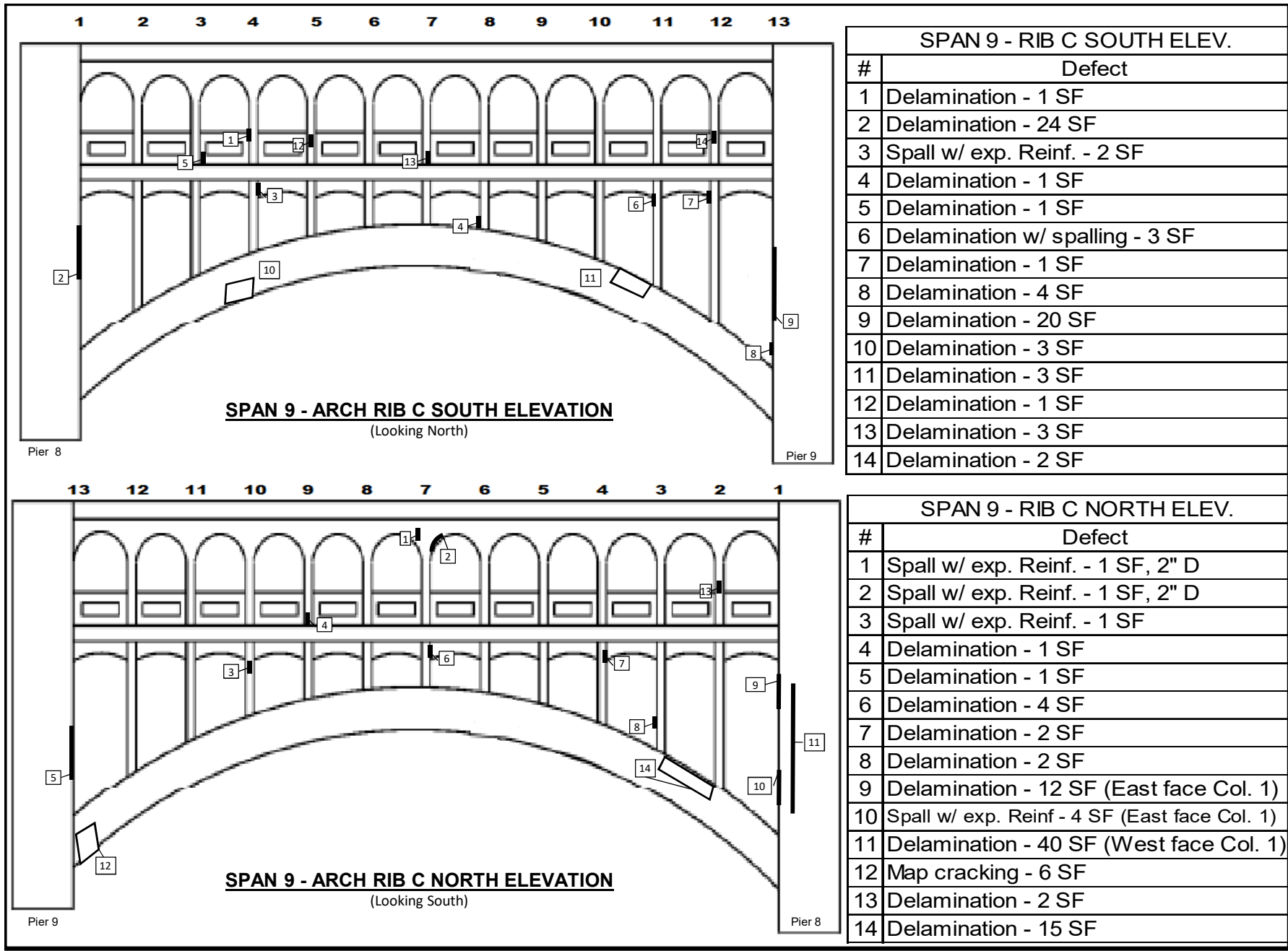


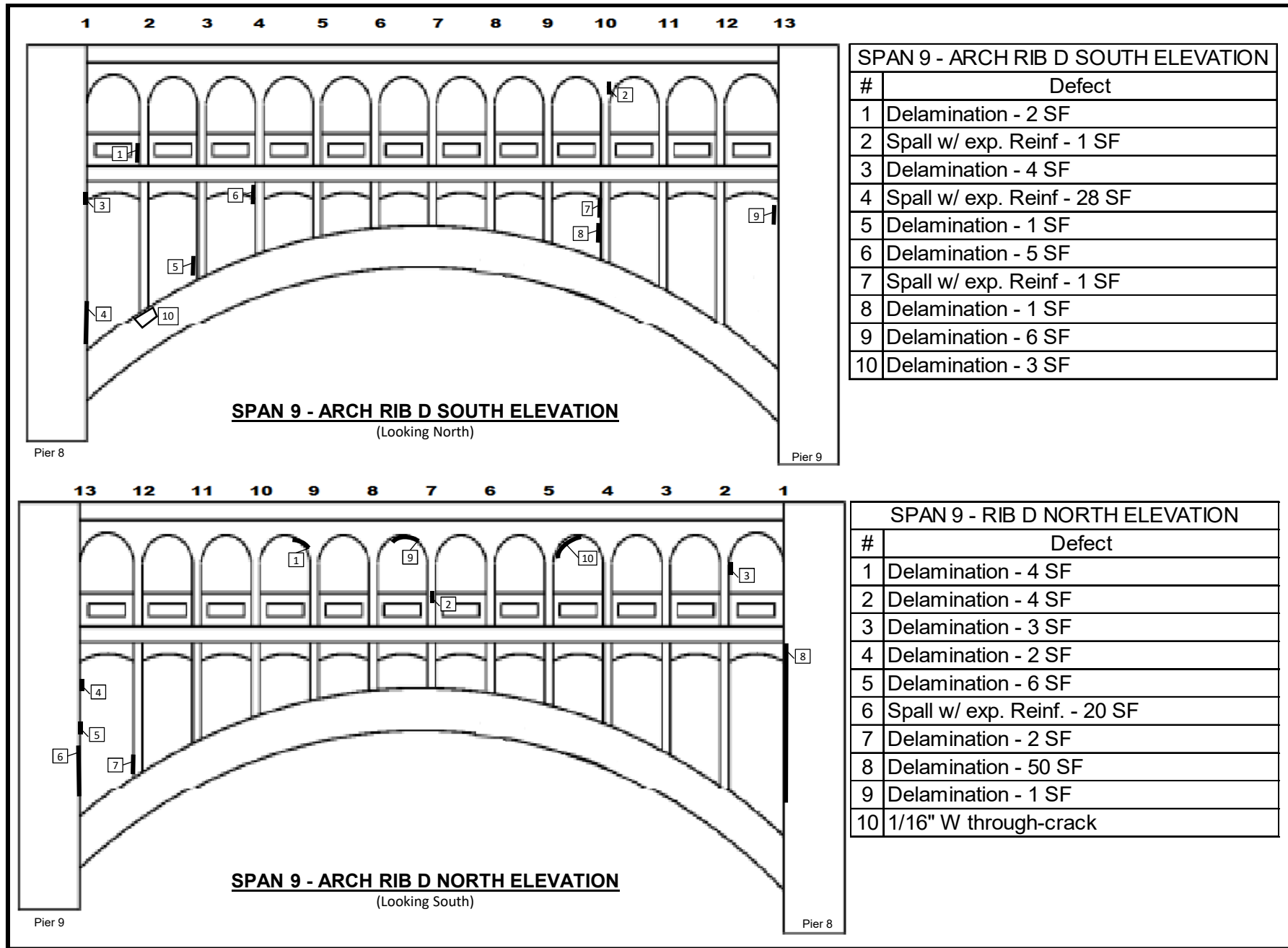


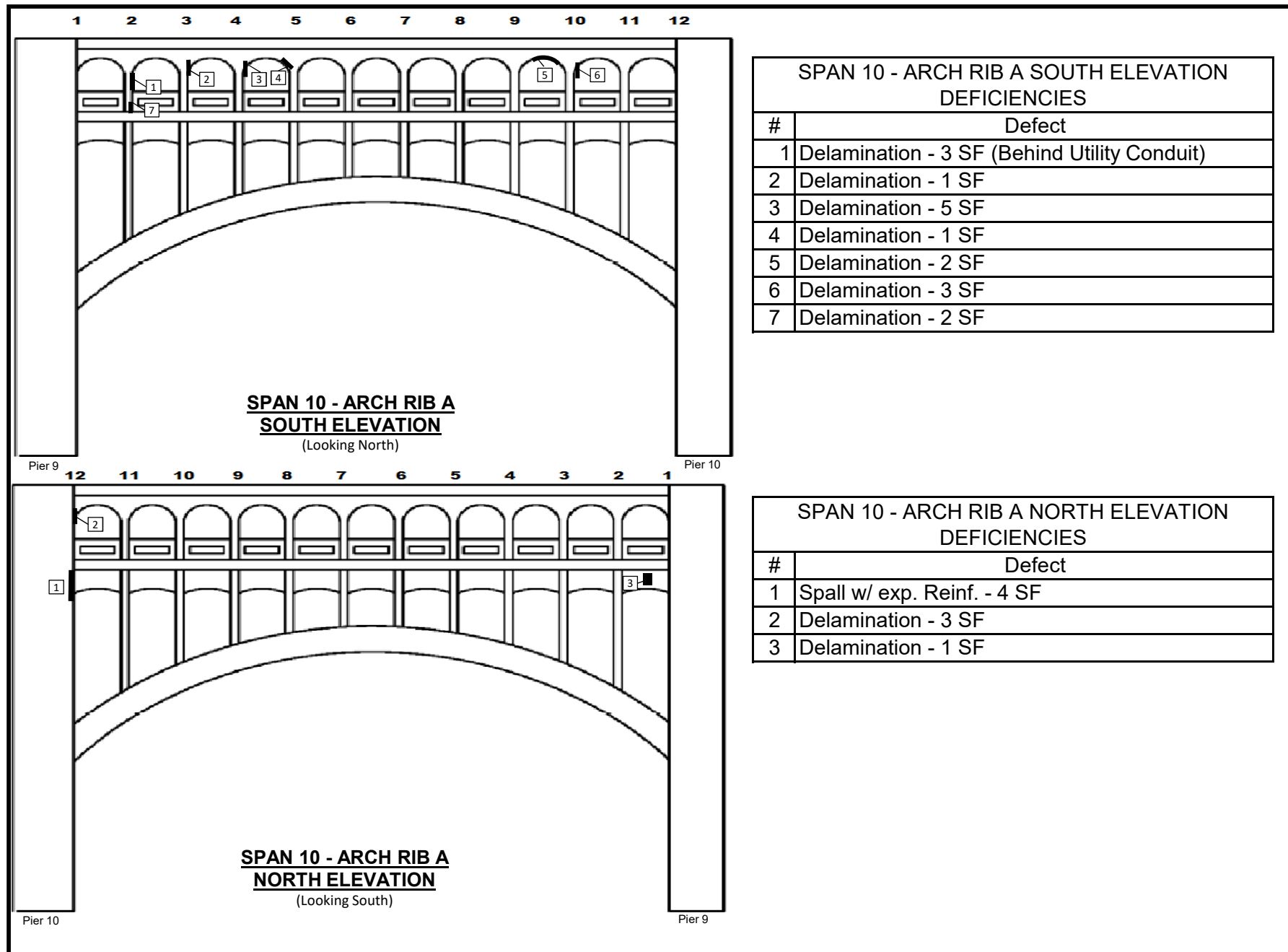






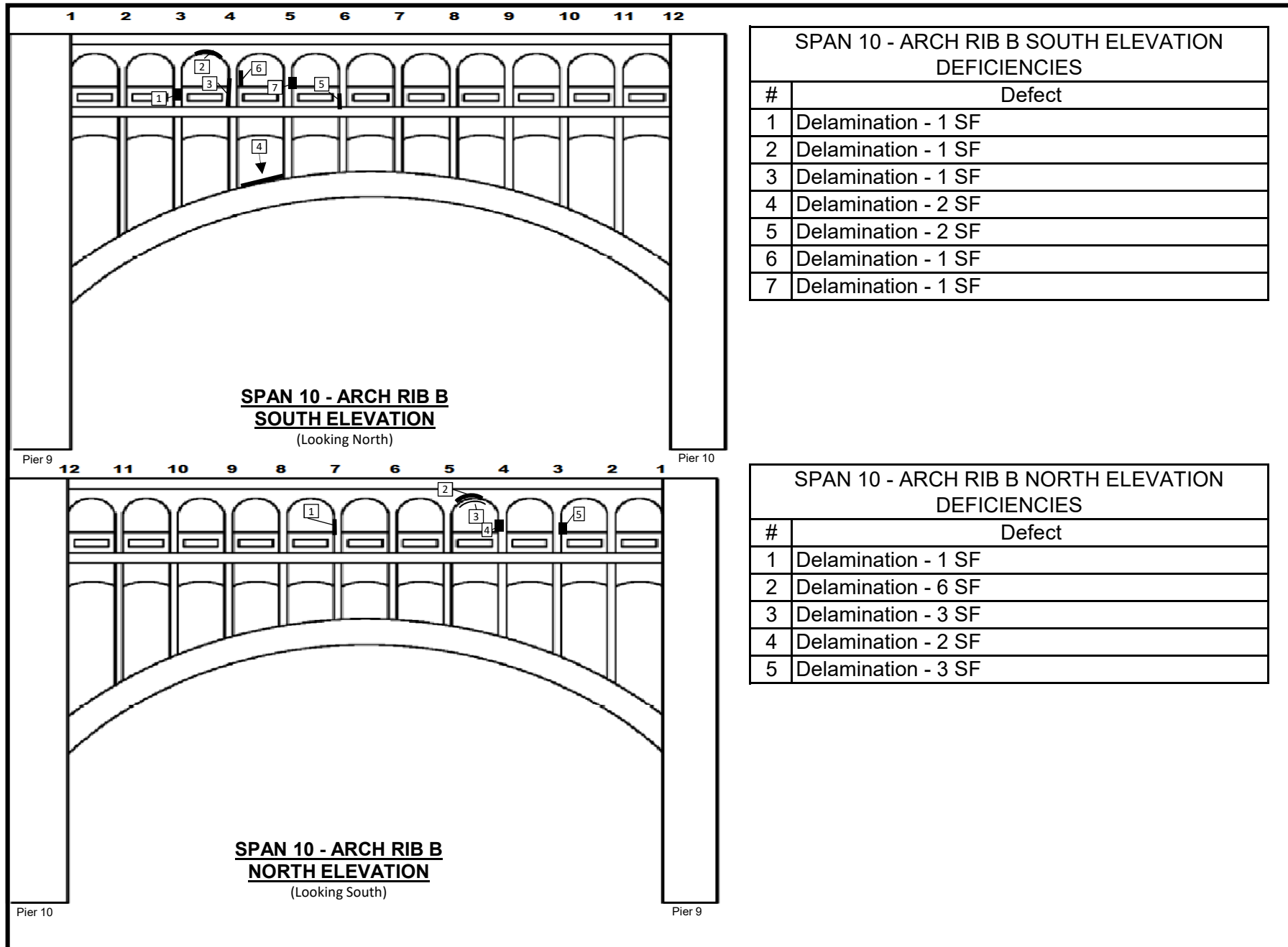


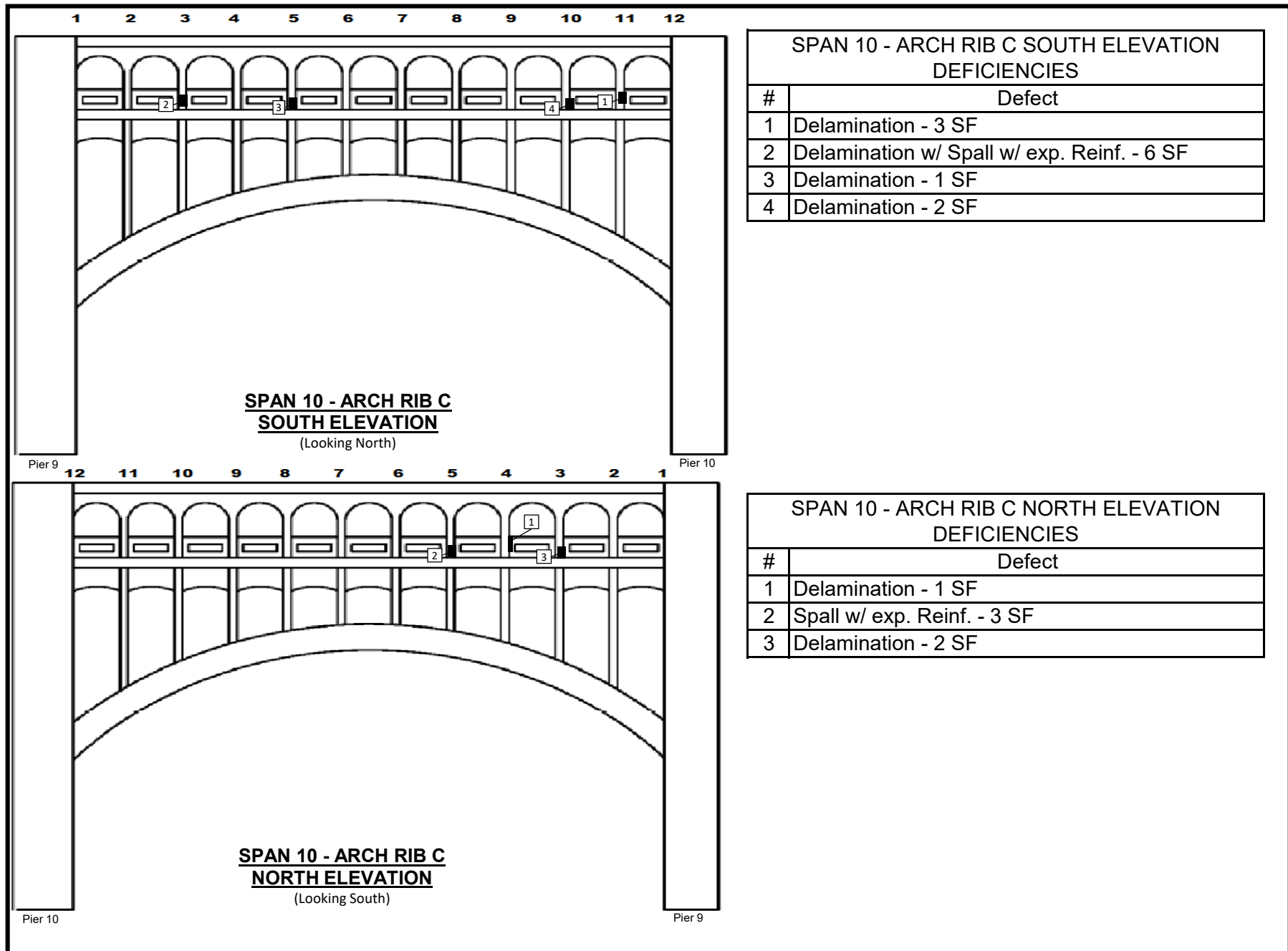


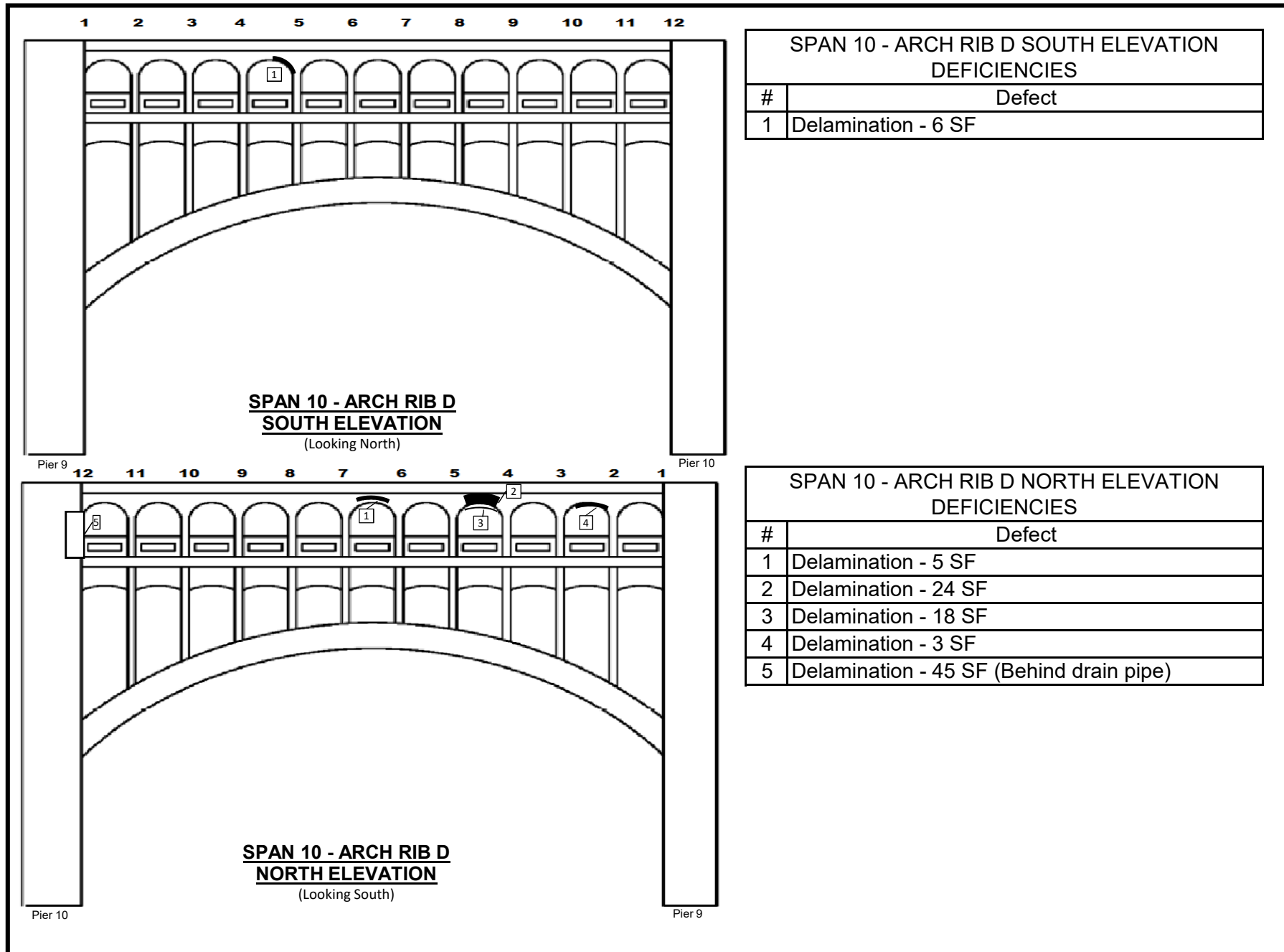


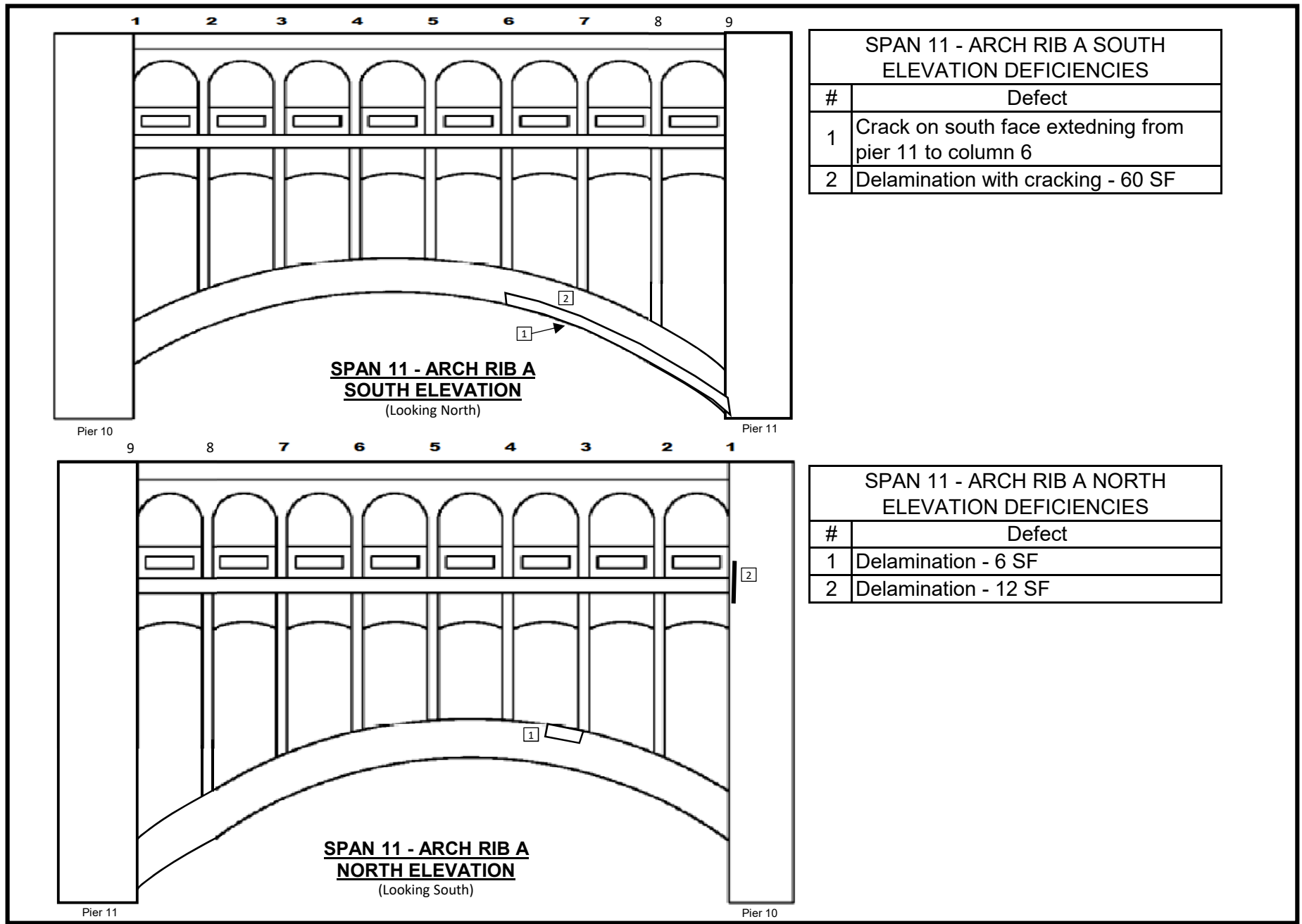
SPAN 10 - ARCH RIB A SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 3 SF (Behind Utility Conduit)
2	Delamination - 1 SF
3	Delamination - 5 SF
4	Delamination - 1 SF
5	Delamination - 2 SF
6	Delamination - 3 SF
7	Delamination - 2 SF

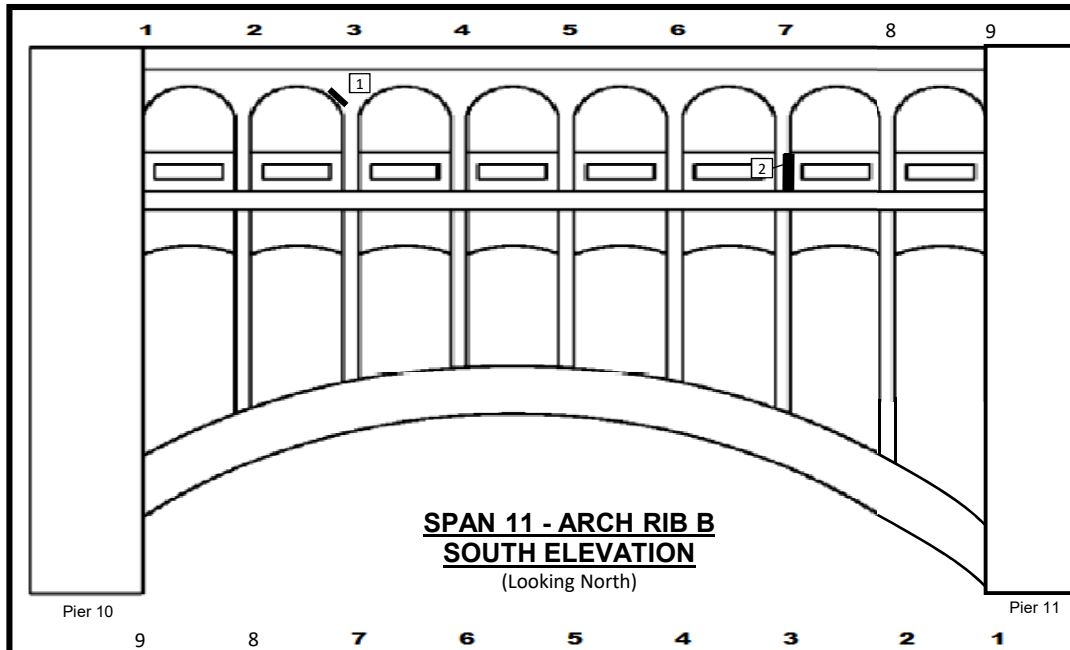
SPAN 10 - ARCH RIB A NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 4 SF
2	Delamination - 3 SF
3	Delamination - 1 SF



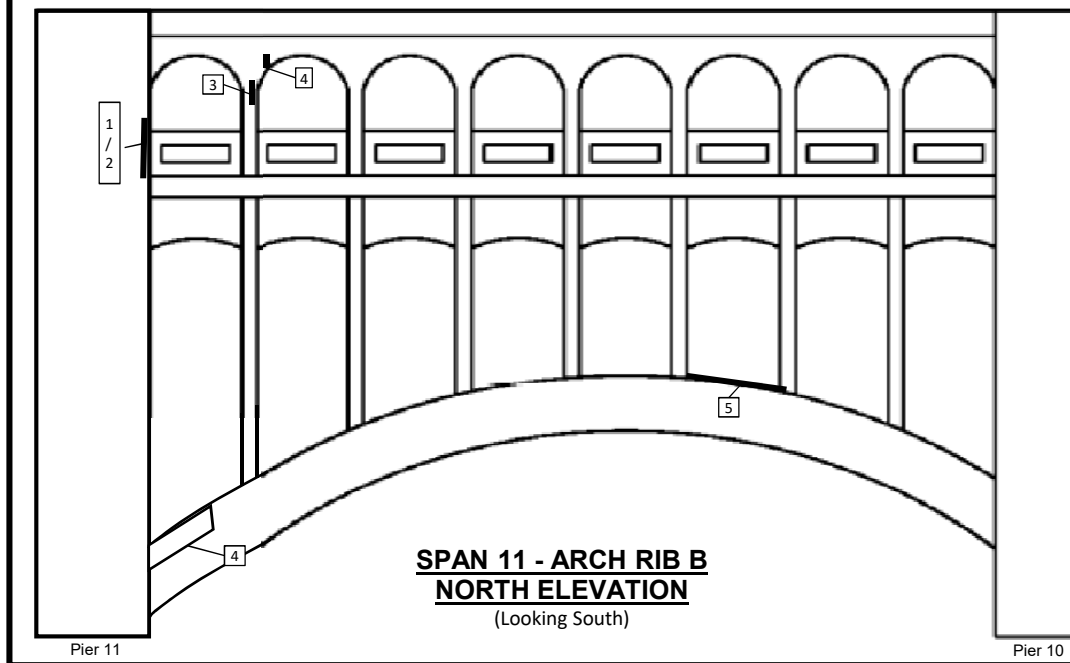




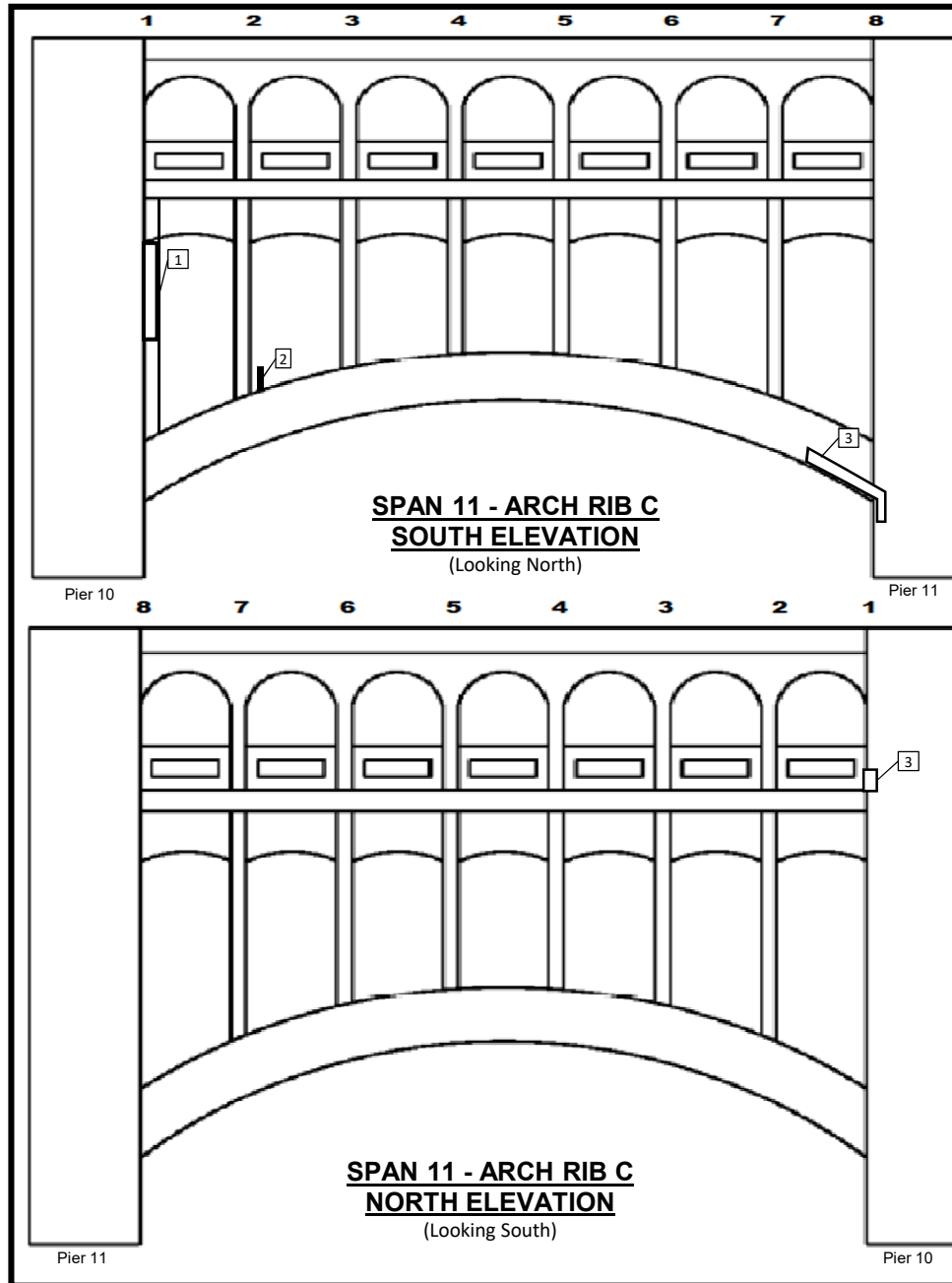




SPAN 11 - ARCH RIB B SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 1 SF
2	Spall w/ exp. Reinf. - 1 SF, 3" D

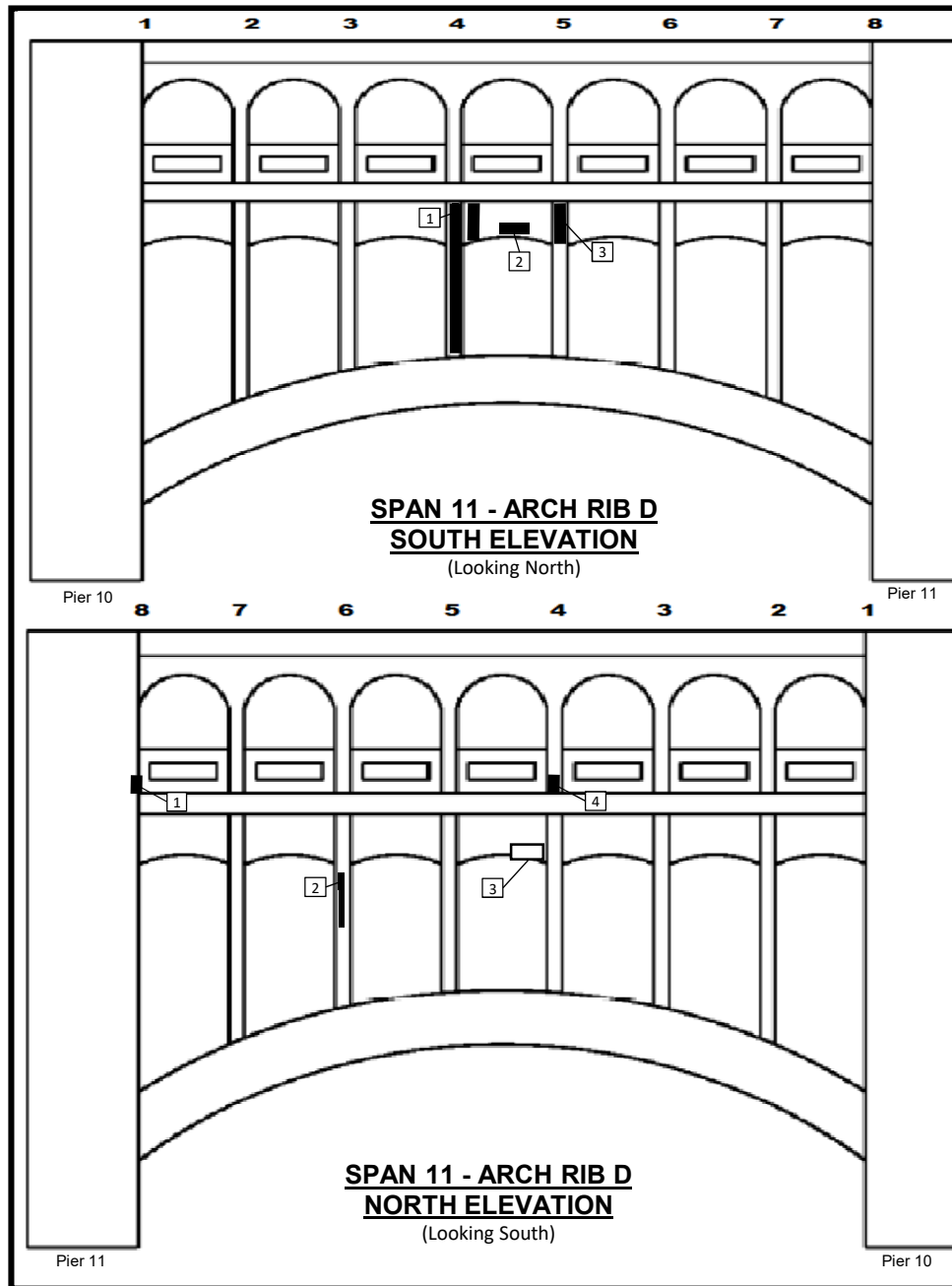


SPAN 11 - ARCH RIB B NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp Reinf. - 1 SF, 3" D
2	Delamination - 35 SF
3	Delamination - 3 SF
4	Spall - 3 SF, 3" D
5	Delamination - 6 SF
6	Delamination - 4 SF



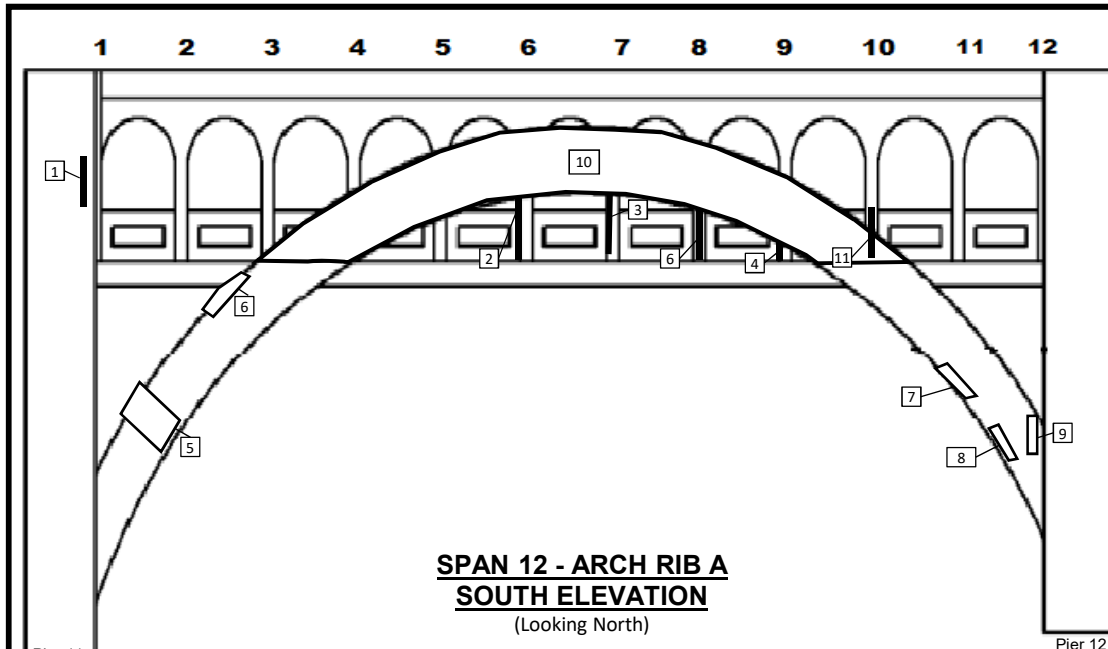
SPAN 11 - ARCH RIB C SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 8 SF
2	Delamination - 1 SF
3	Delamination - 6 SF

SPAN 11 - ARCH RIB C NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Incomplete repair location (no reinf. exp.) - 1 SF



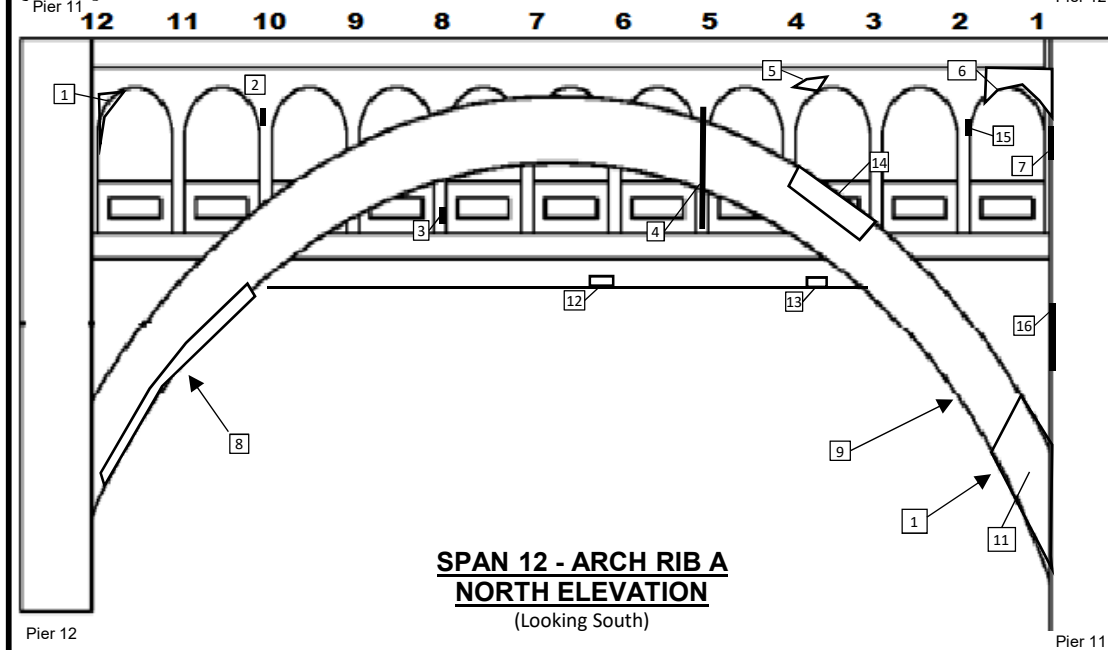
SPAN 11 - ARCH RIB D SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Spall w/ exp. Reinf. - 1.5' W x Full H, 6" D
2	Delamination - 2 SF
3	Delamination - 2 SF

SPAN 11 - ARCH RIB D NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 2 SF
2	Delamination - 2 SF
3	Delamination - 3 SF
4	Delamination - 2 SF

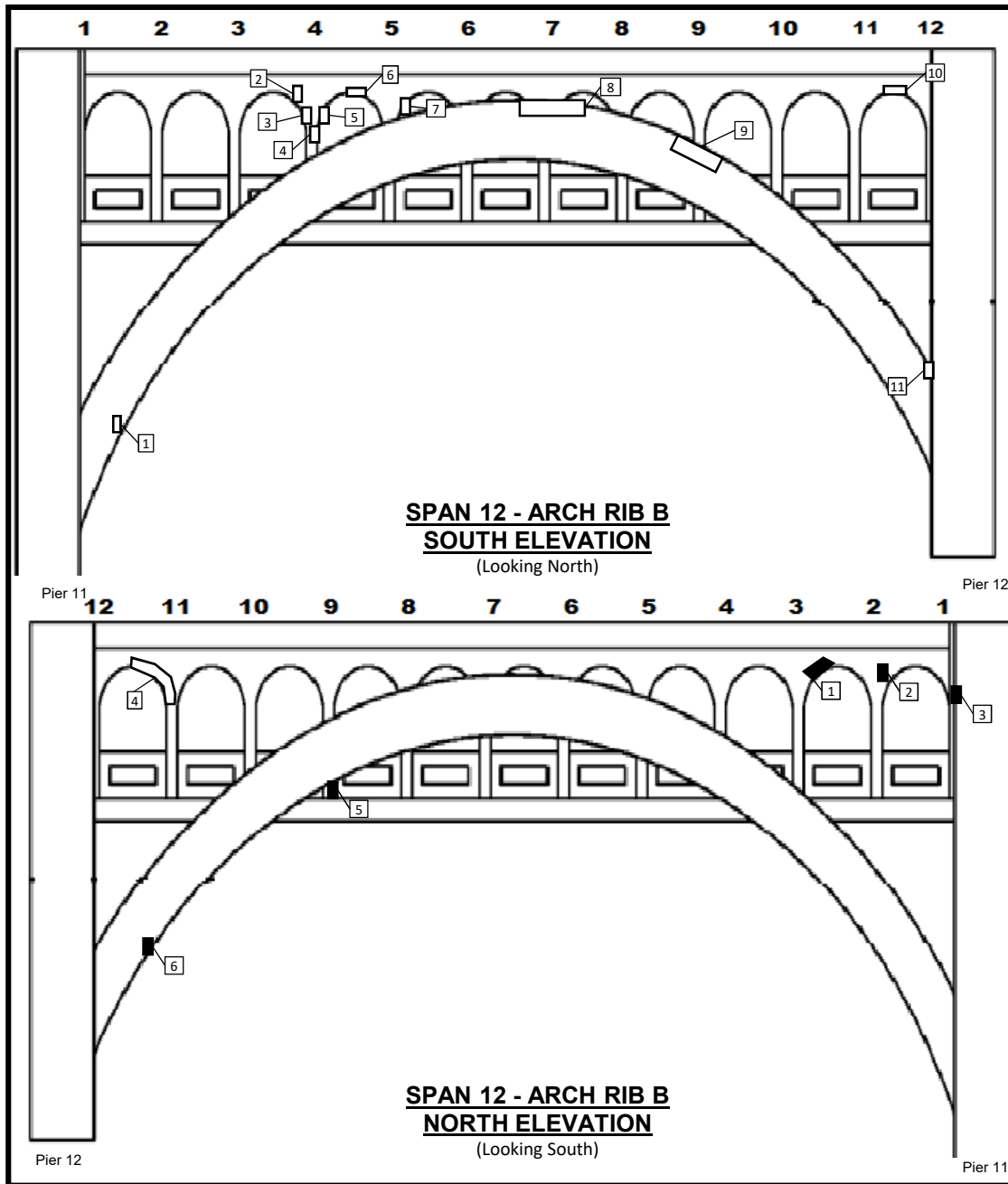


Note: All concrete encased steel hangers sound delaminated due to only 4"± cover between steel hanger and outside face of concrete.

SPAN 12 - RIB A SOUTH ELEVATION DEFICIENCIES			
#	Defect	#	Defect
1	Spall w/ exp. Reinf. - 3 SF, 4" D	7	Delamination - 2 SF
		8	Delamination - 3 SF
2	Delamination w/ 1/8" W Crack - 10 SF	9	Delamination - 1 SF
3	Delamination w/ up to 1/2" W Crack - 15 SF	10	South face of arch is delaminated (90%) with widespread cracking near the bases. 1/2" W crack in the delaminated surface.
4	Delamination - 8 SF		
5	Delamination - 10 SF		
6	Delamination - 6 SF		
11	Delamination - 4 SF		



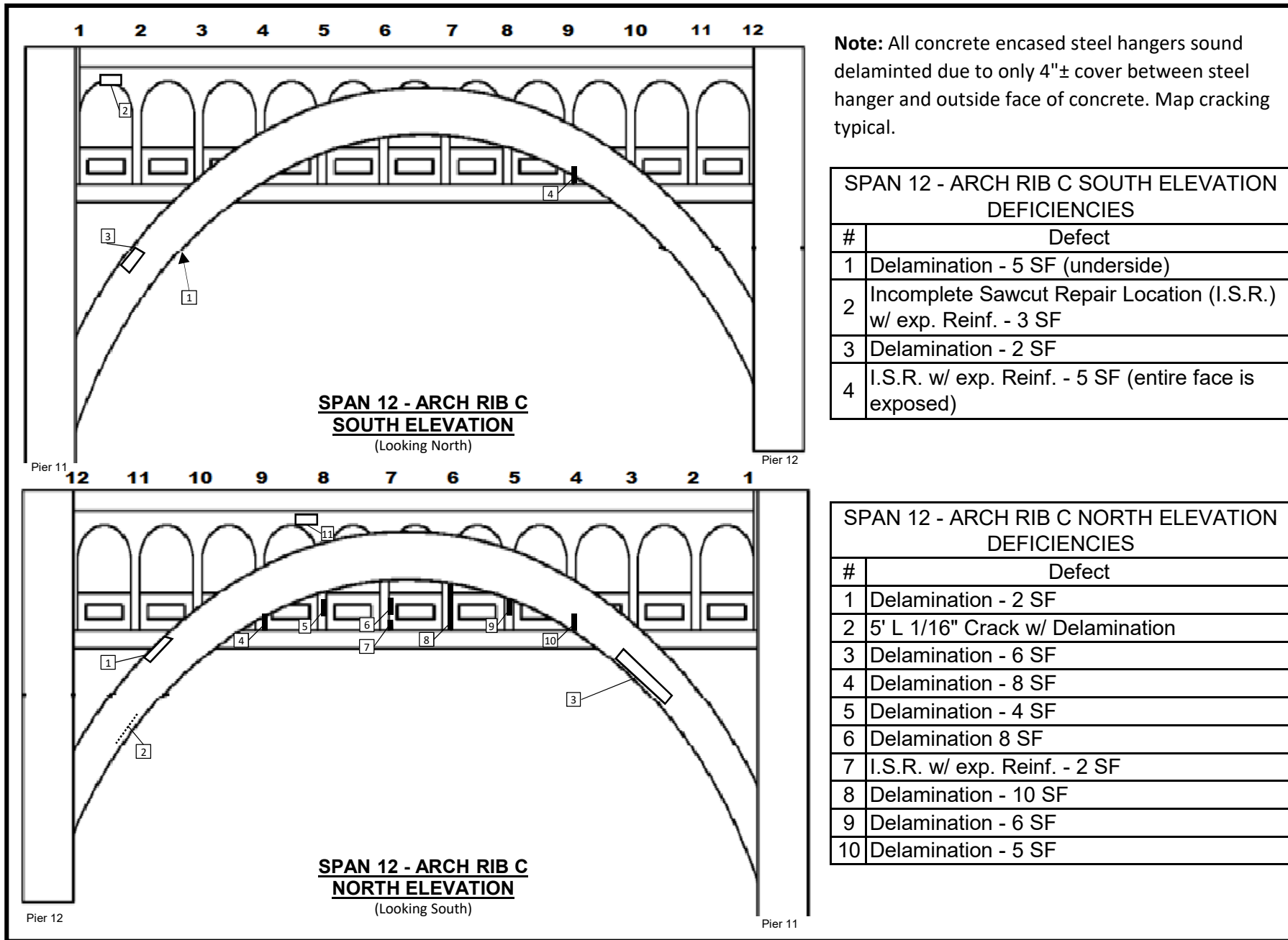
SPAN 12 - RIB A NORTH ELEVATION DEFICIENCIES			
#	Defect	#	Defect
1	Spall w/ exp. Reinf - 12 SF, 6" D	9	Delamination - 5 SF (U.o.A)
2	Spall w/ exp. Reinf - 1 SF, 2" D	10	Delamination - 18 SF (U.o.A)
3	Spall w/ exp. Reinf - 1 SF, 2" D	11	Delamination - 20 SF
4	Delamination - 8 SF	12	Spall - 1 SF
5	Spall w/ exp. Reinf - 1 SF, 2" D	13	Spall - 1 SF
		14	Delamination - 15 SF
6	Entire face of arch is spalling and delaminated, up to 3" D	15	Spall w/ exp. Reinf. - 1 SF
7	Spall - 8 SF, 4" D (on pier)	16	Delamination w/ Spall w/ exp. Reinf. - 4 SF (on pier)

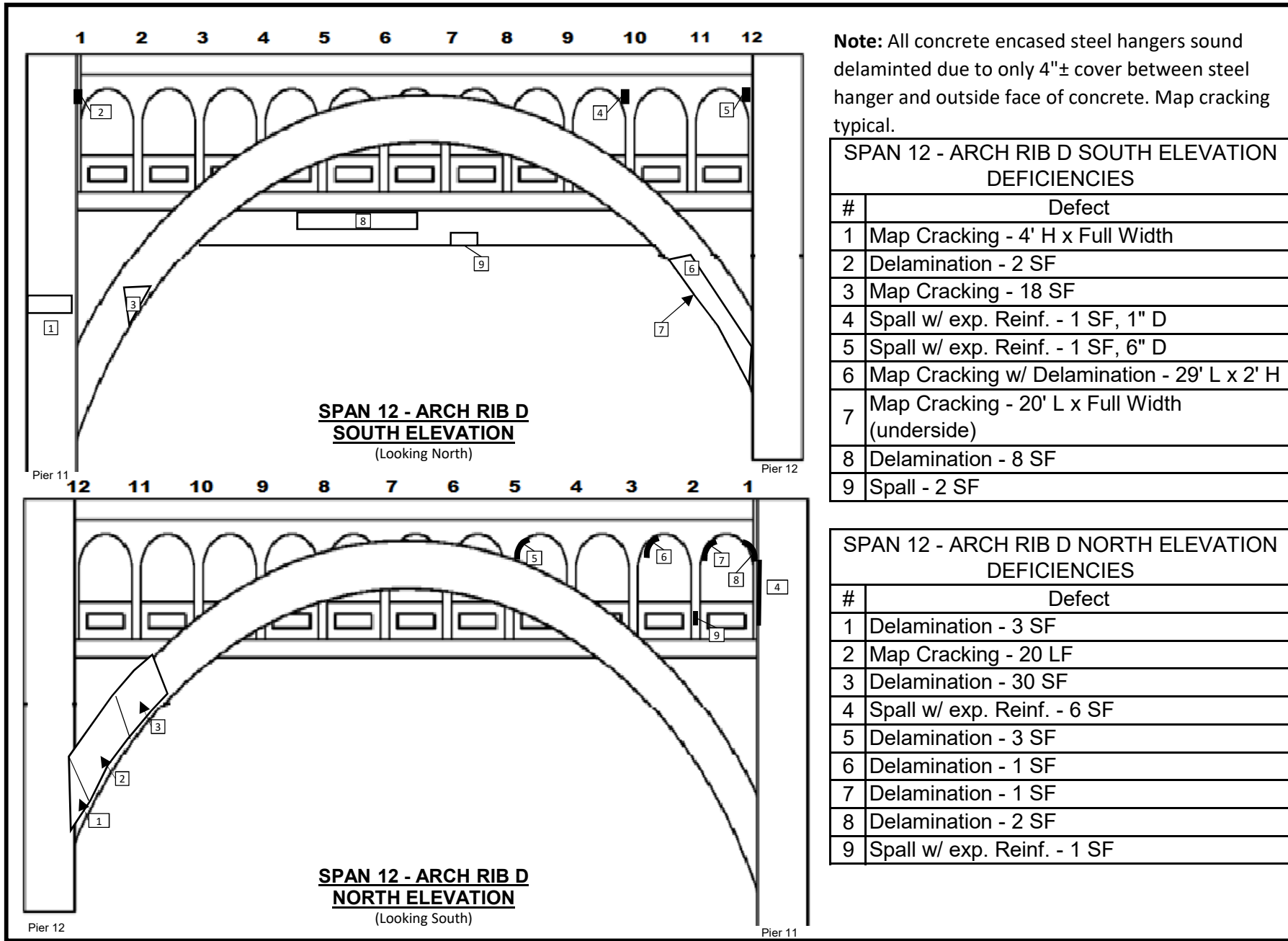


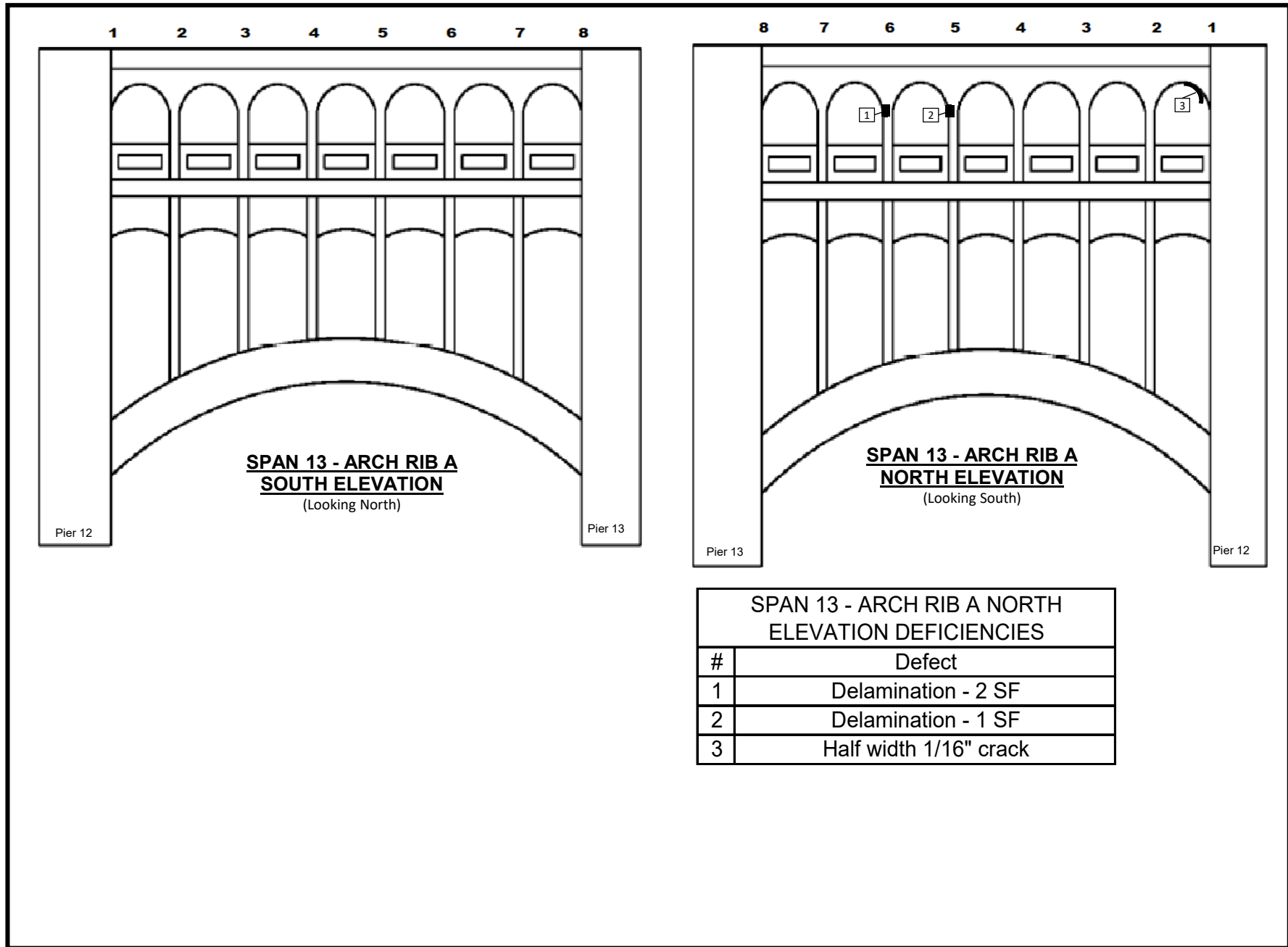
Note: All concrete encased steel hangers sound delaminated due to only 4"± cover between steel hanger and outside face of concrete.

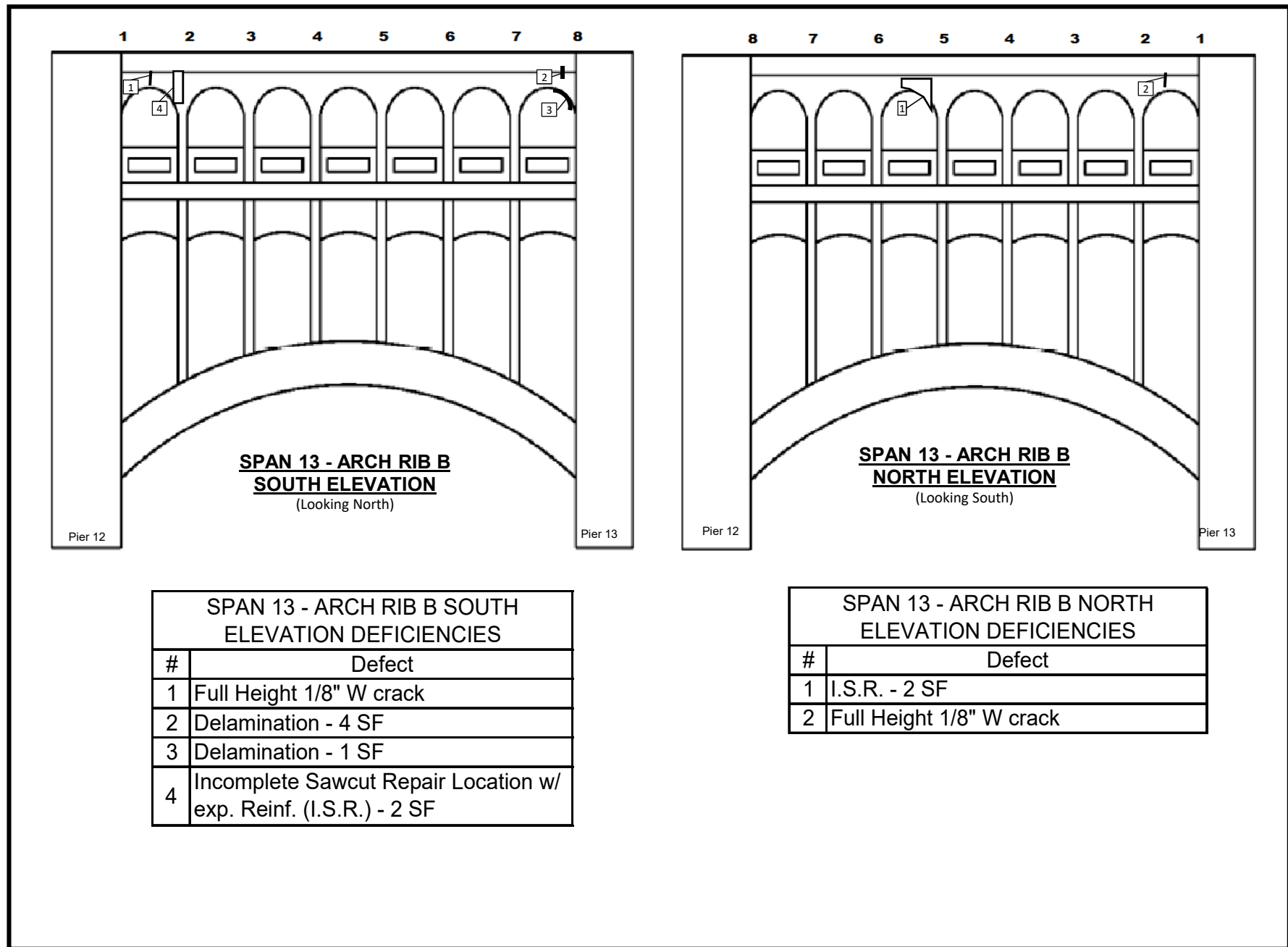
SPAN 12 - ARCH RIB B SOUTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 1 SF
2	Incomplete Sawcut Repair (I.S.R.) Location - 1 SF
3	I.S.R. - 1 SF
4	I.S.R. - 1 SF
5	I.S.R. - 1 SF
6	I.S.R. - 1 SF
7	I.S.R. - 1 SF
8	I.S.R. - 4 SF
9	I.S.R. - 3 SF
10	I.S.R. - 4 SF
11	Delamination - 1 SF

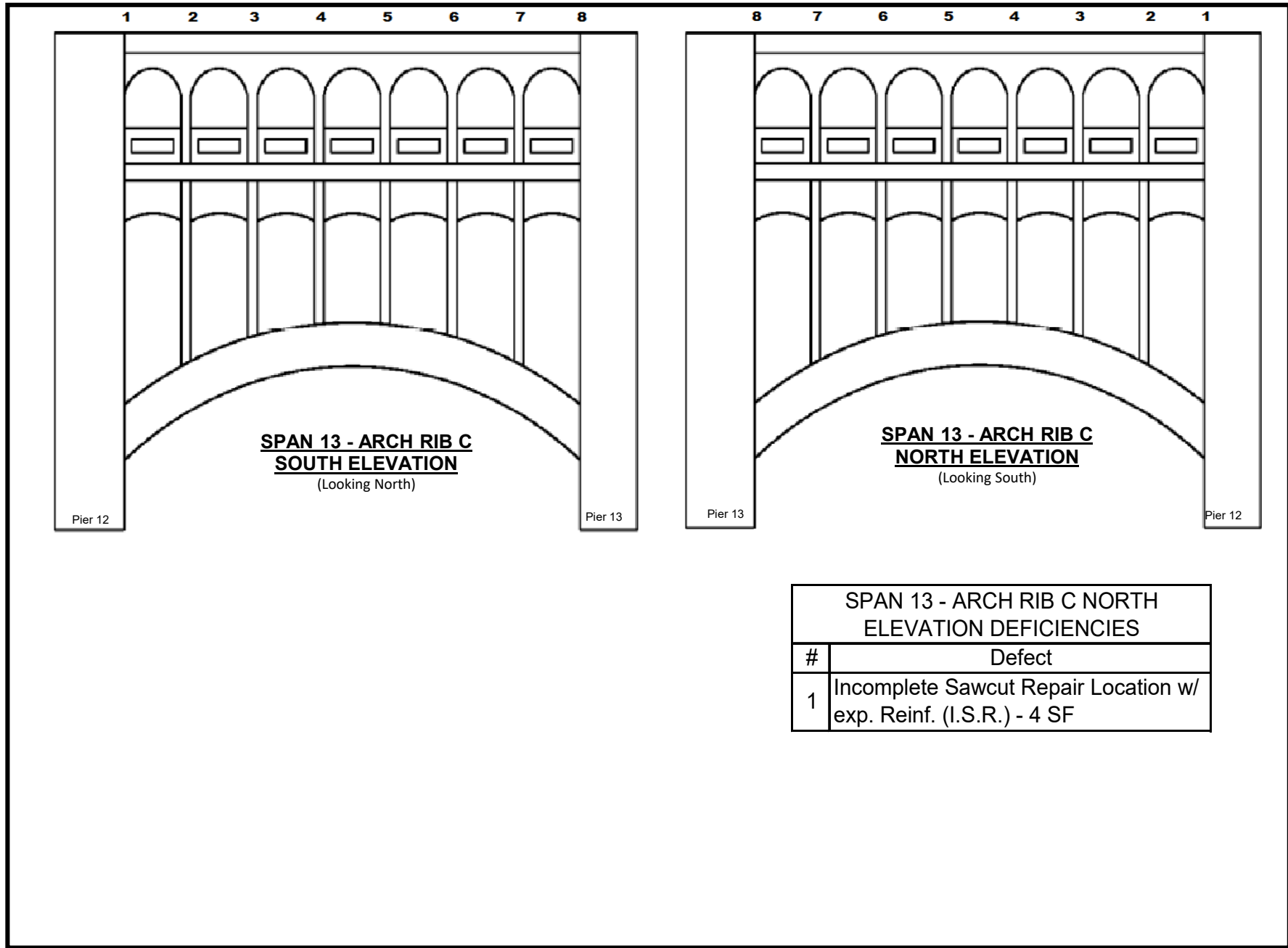
SPAN 12 - ARCH RIB B NORTH ELEVATION DEFICIENCIES	
#	Defect
1	Delamination - 1.5 SF
2	Delamination - 1 SF
3	Delamination - 1 SF
4	Delamination - 1 SF
5	Delamination - 9 SF
6	Delamination - 6 SF

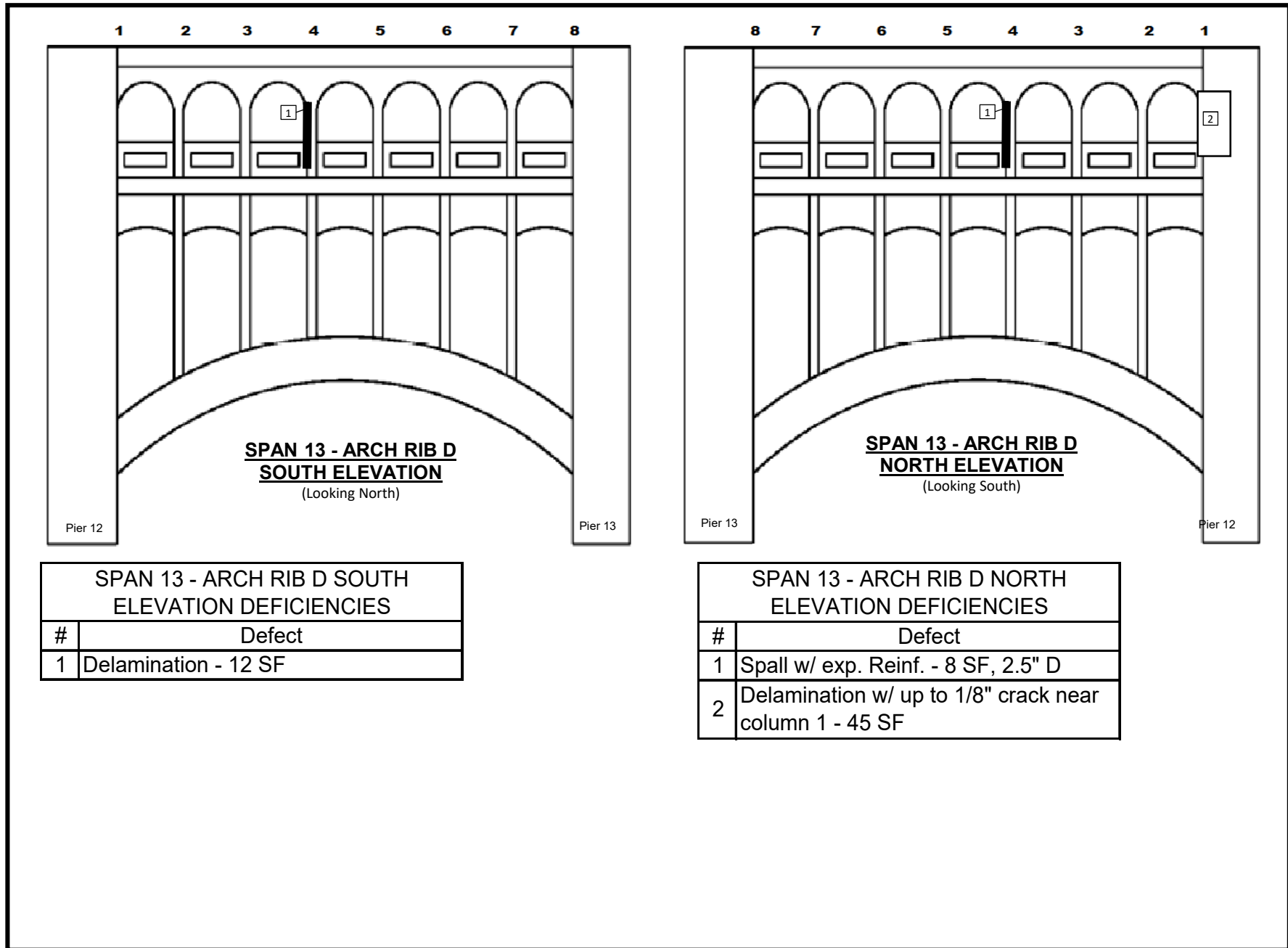


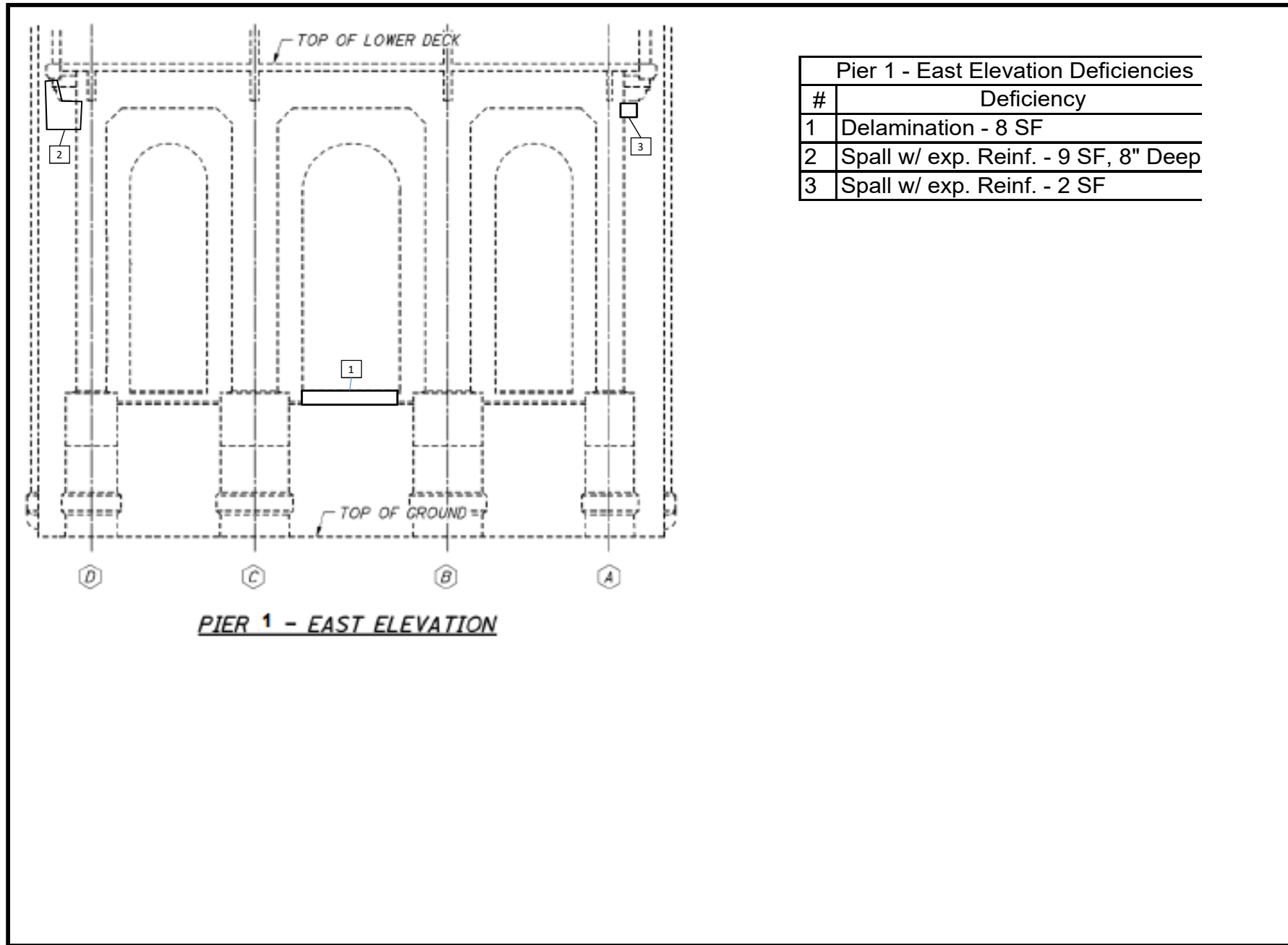


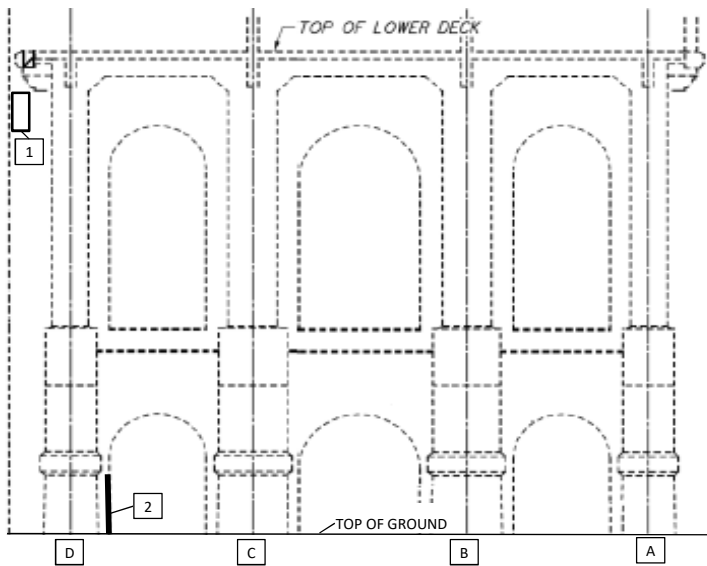






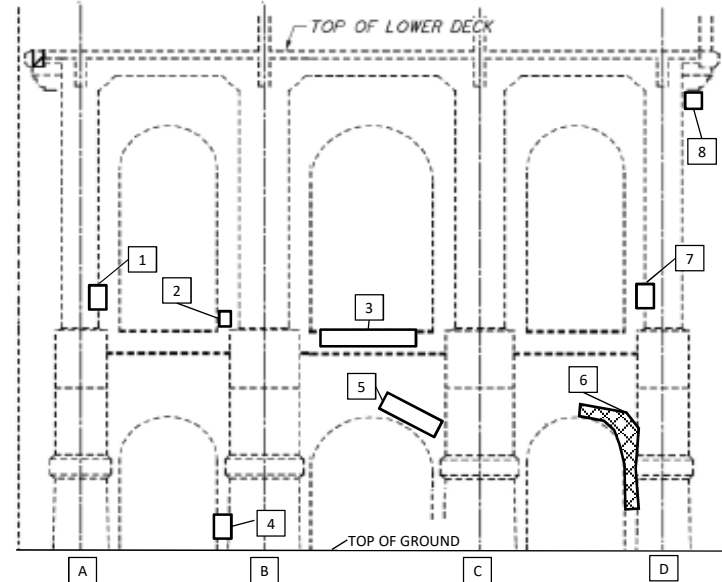






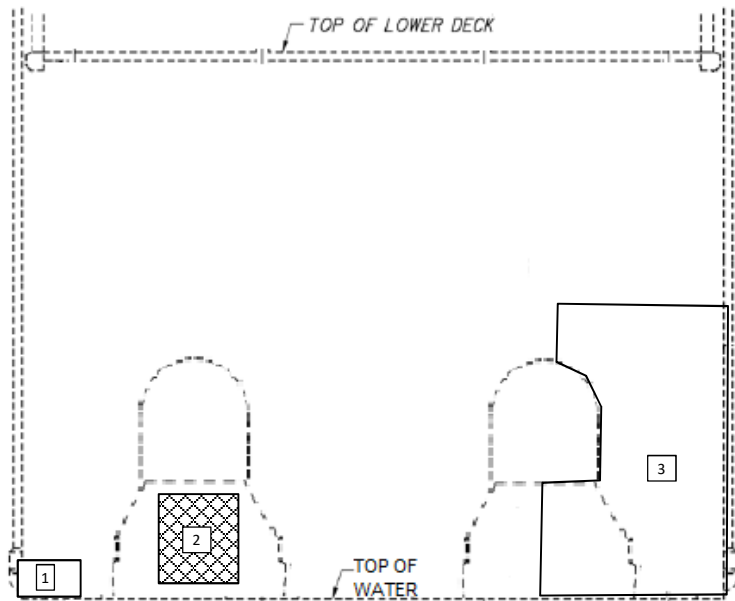
PIER 2 - EAST ELEVATION

Pier 2 - East Elevation Deficiencies	
#	Deficiency
1	Spall - 6 SF
2	Map Cracking - 15 SF (inside face)



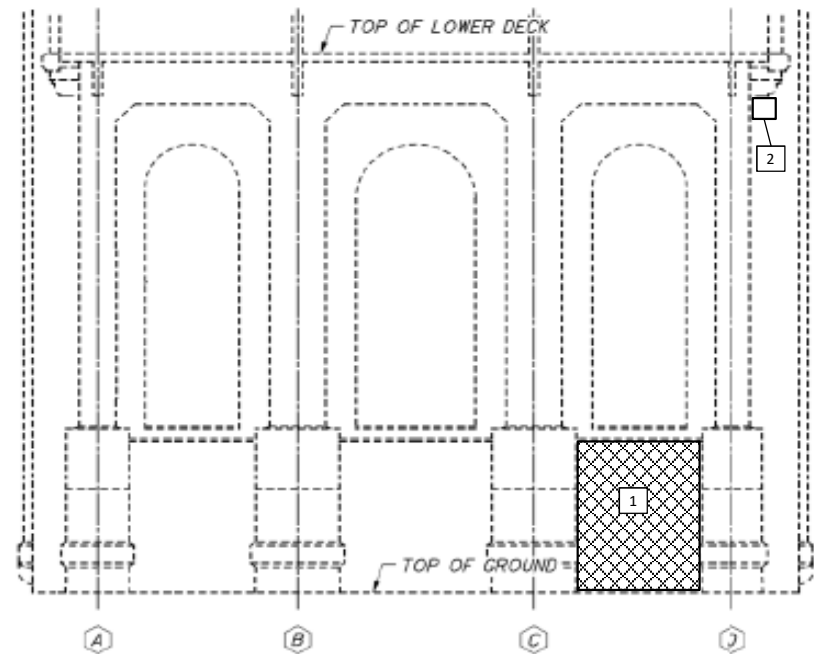
PIER 2 - WEST ELEVATION

Pier 2 - West Elevation Deficiencies	
#	Deficiency
1	Delamination - 4 SF (south inner face)
2	Delamination - 1 SF
3	Delamination - 20 SF
4	Delamination - 4 SF
5	Delamination - 10 SF
6	Map Cracking
7	Delamination - 9 SF (north inner face)
8	Spall w/ exp. Reinf. - 4 SF, 4" Deep



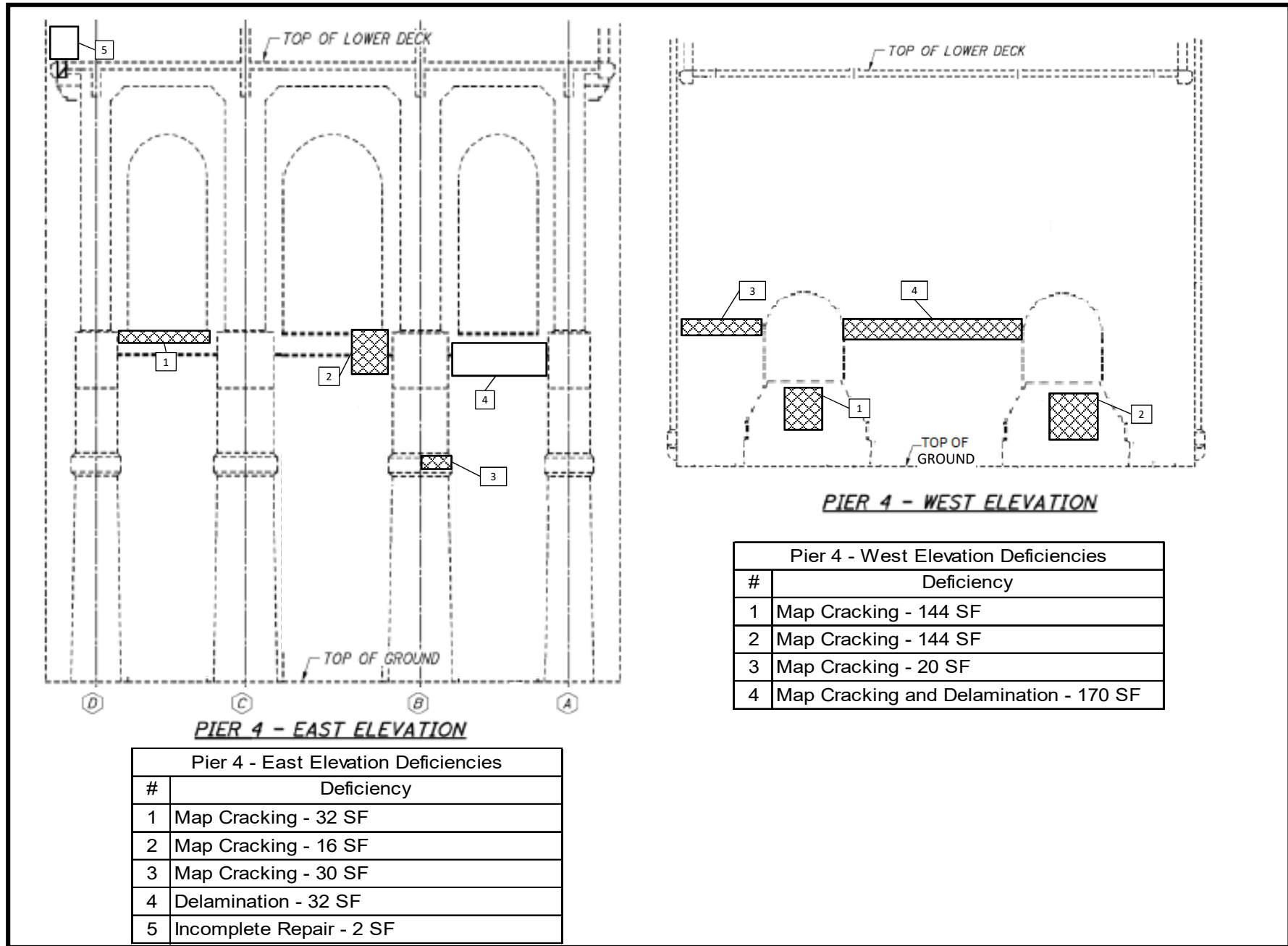
PIER 3 - EAST ELEVATION

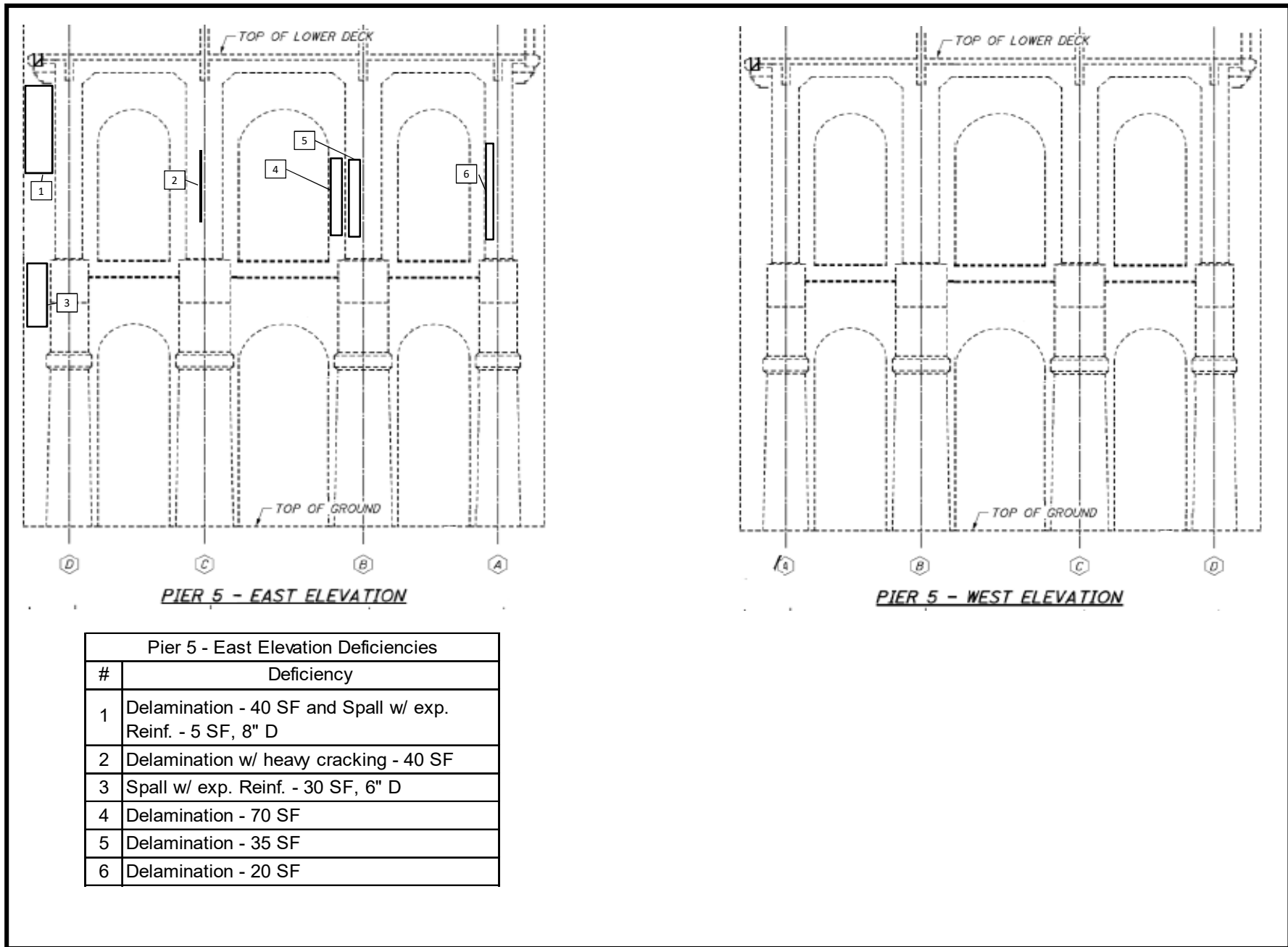
Pier 3 - East Elevation Deficiencies	
#	Deficiency
1	Corner Spall
2	Map Cracking - 144 SF
3	Vine growth covering concrete

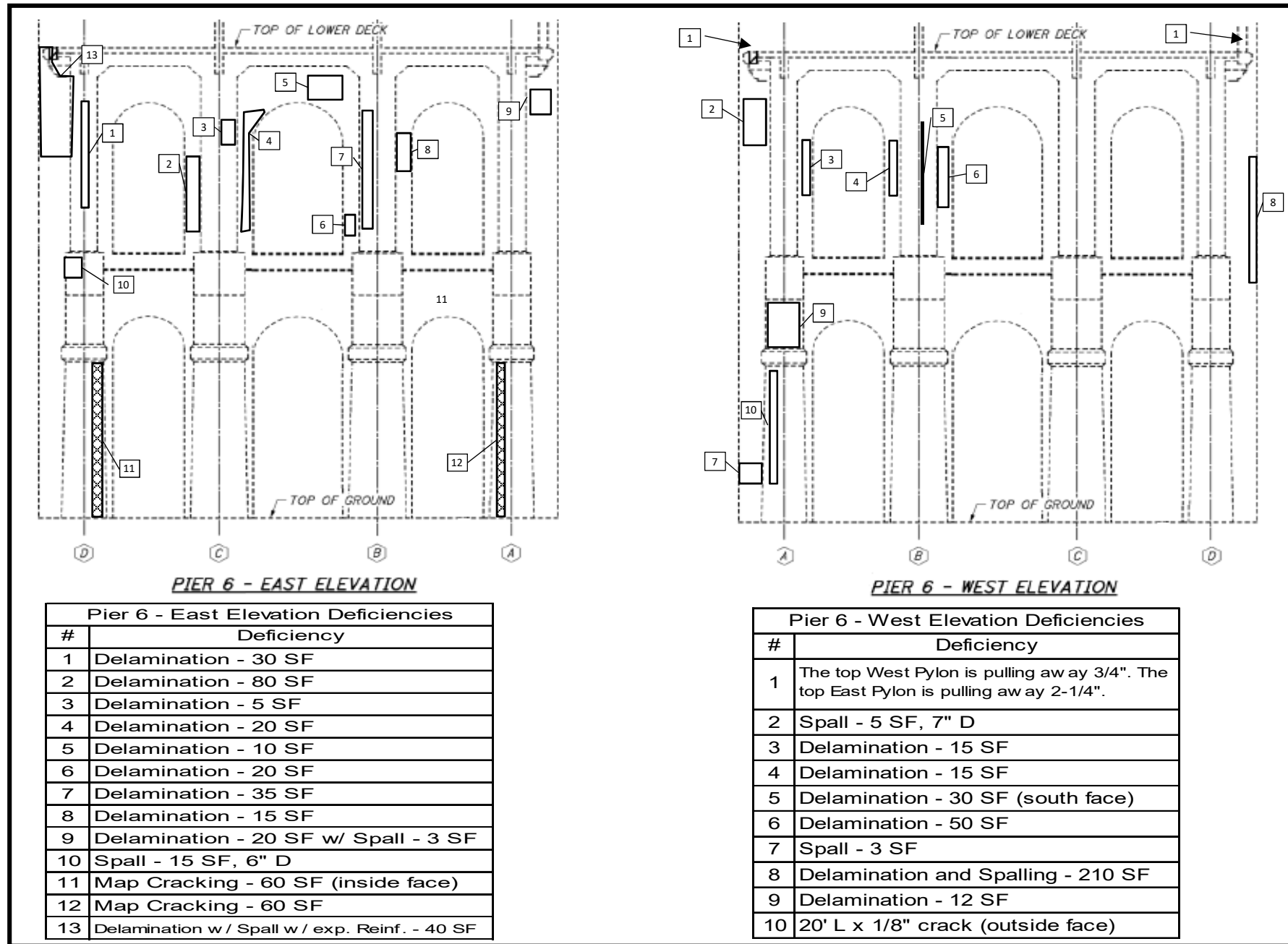


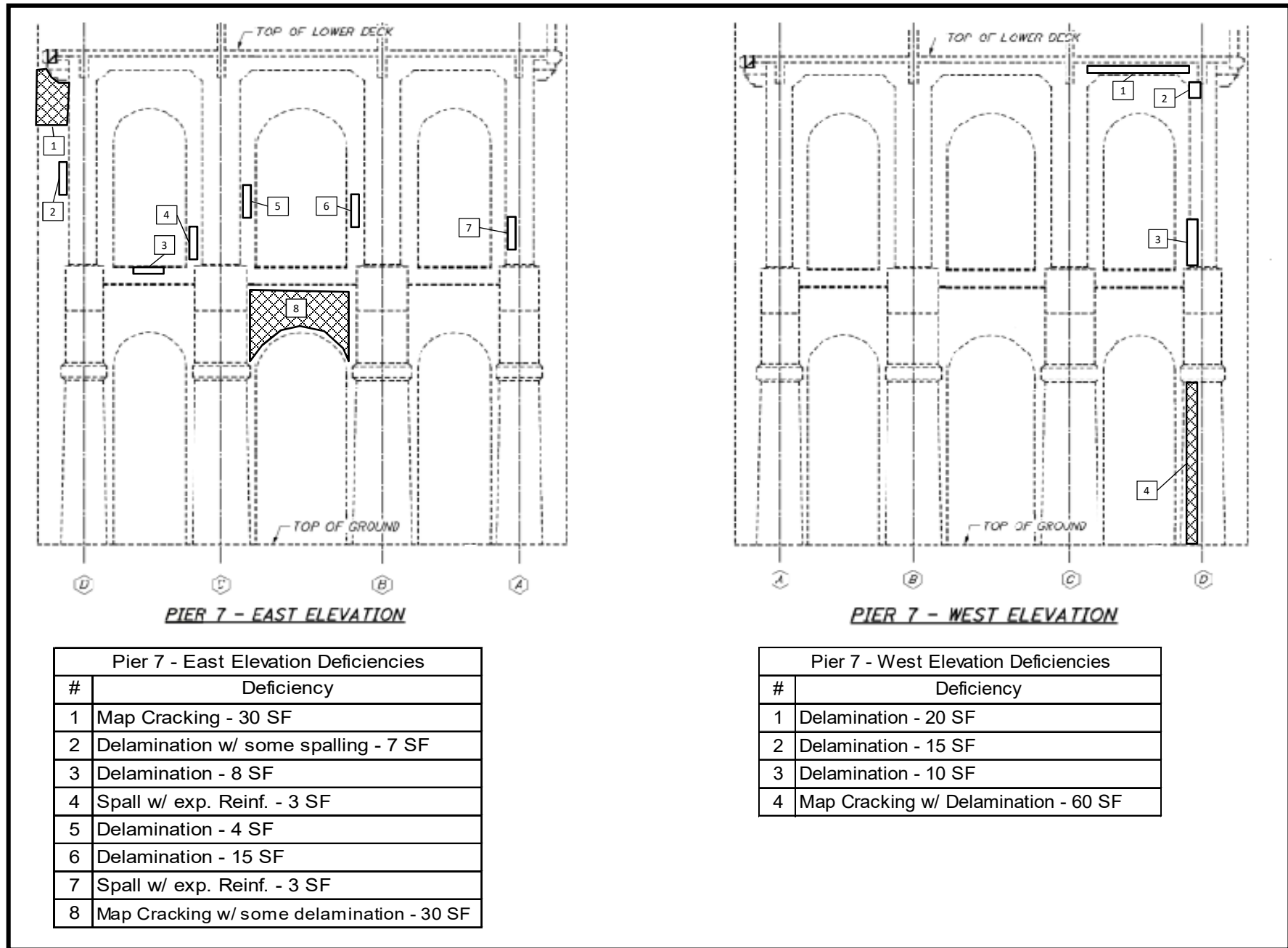
PIER 3 - WEST ELEVATION

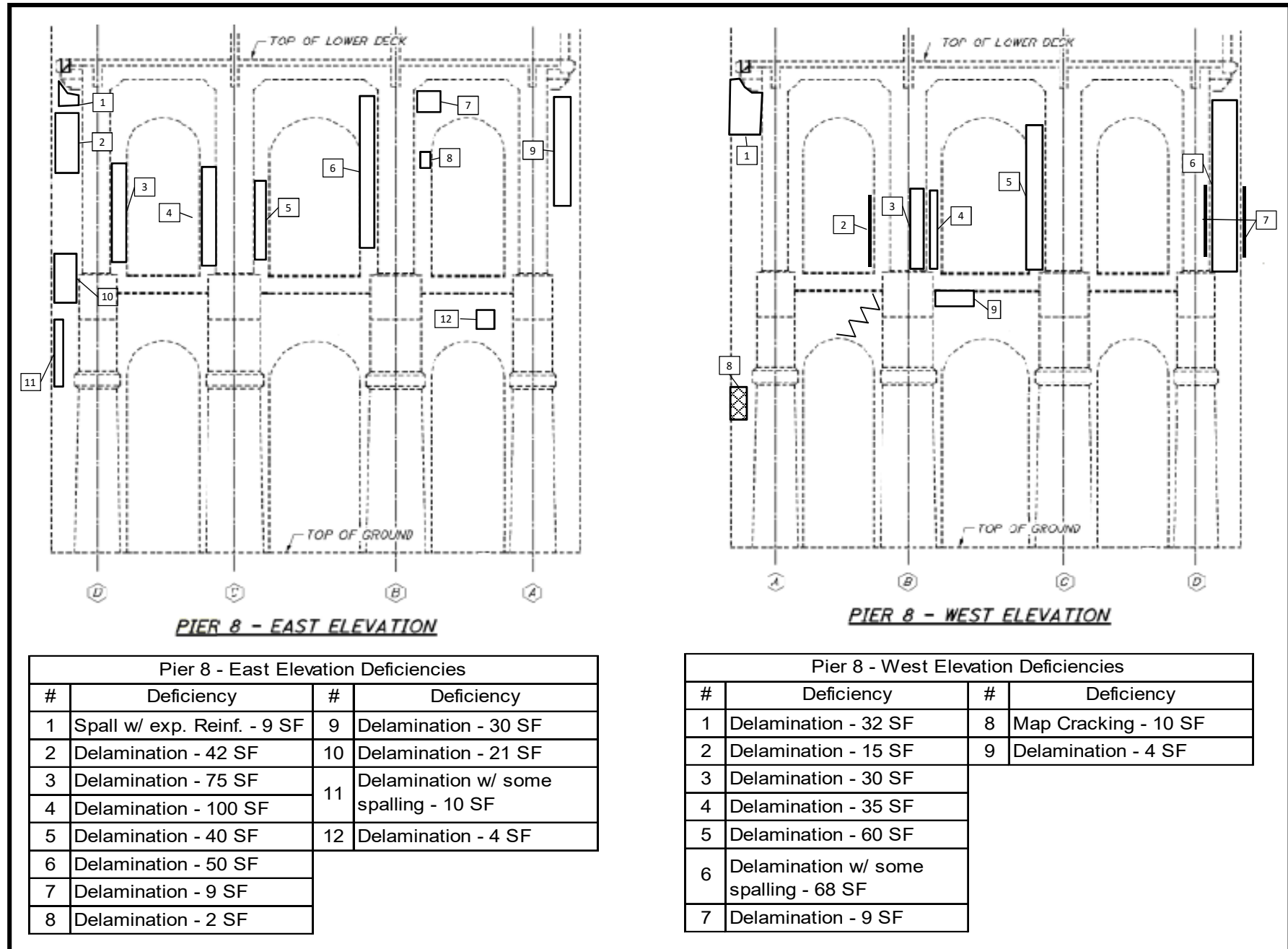
Pier 3 - West Elevation Deficiencies	
#	Deficiency
1	Map Cracking
2	Spall - 1 SF

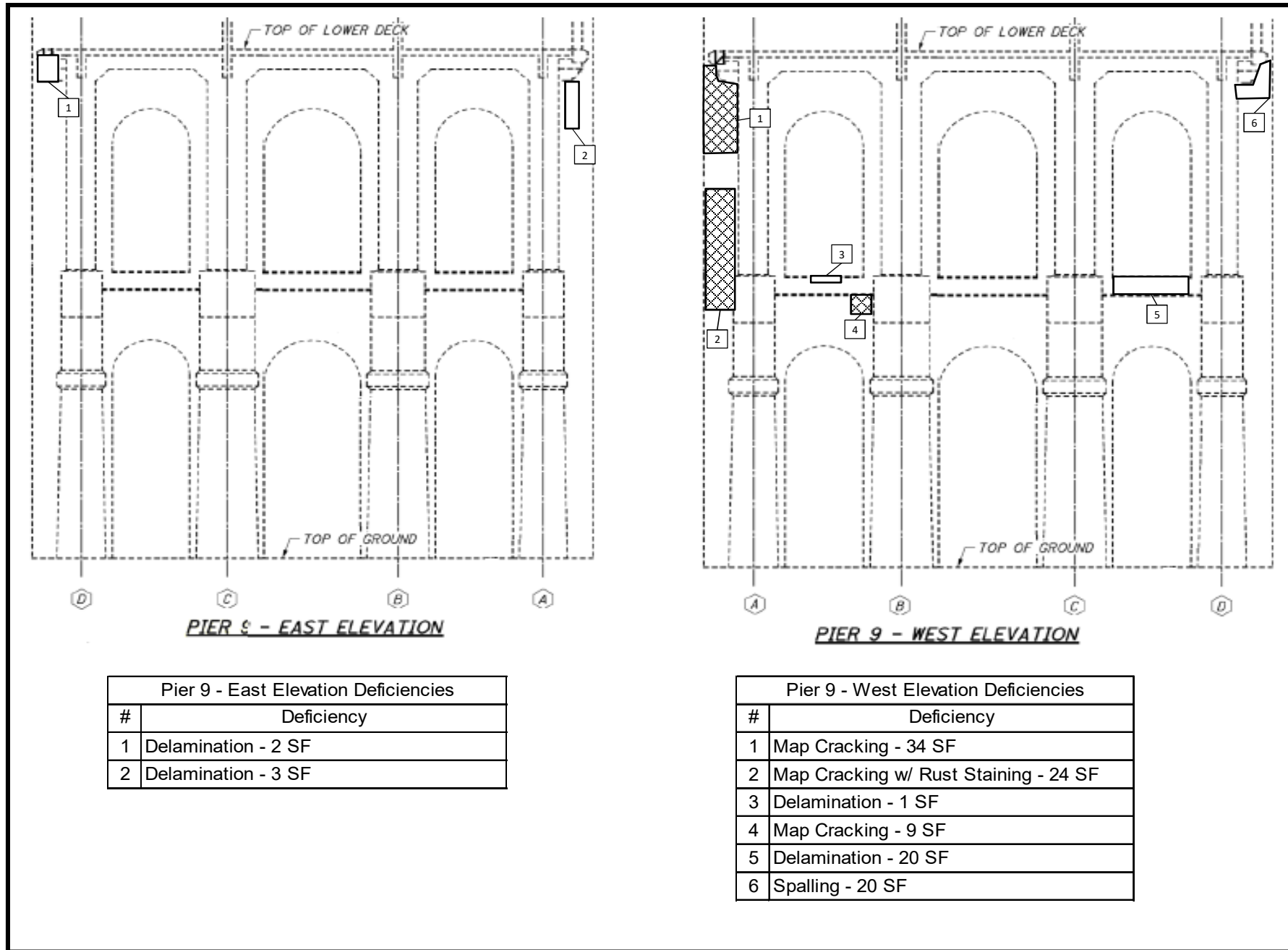


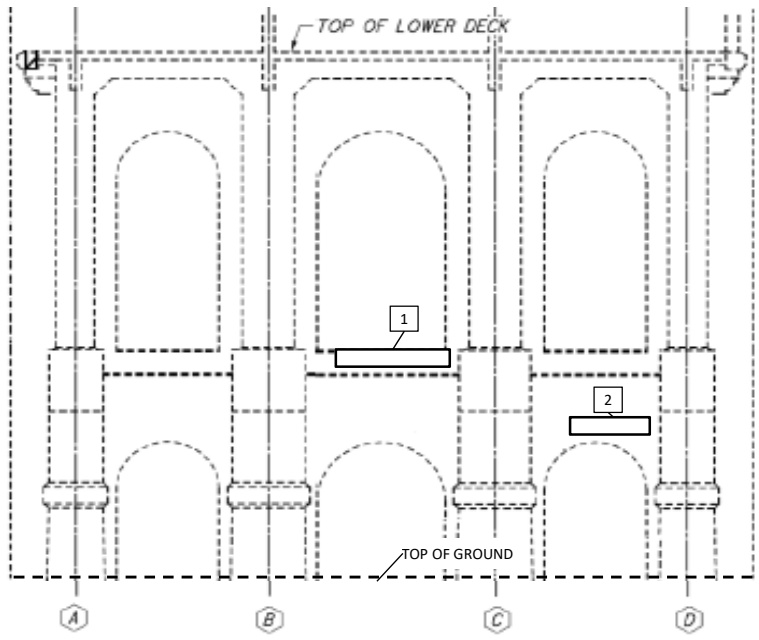




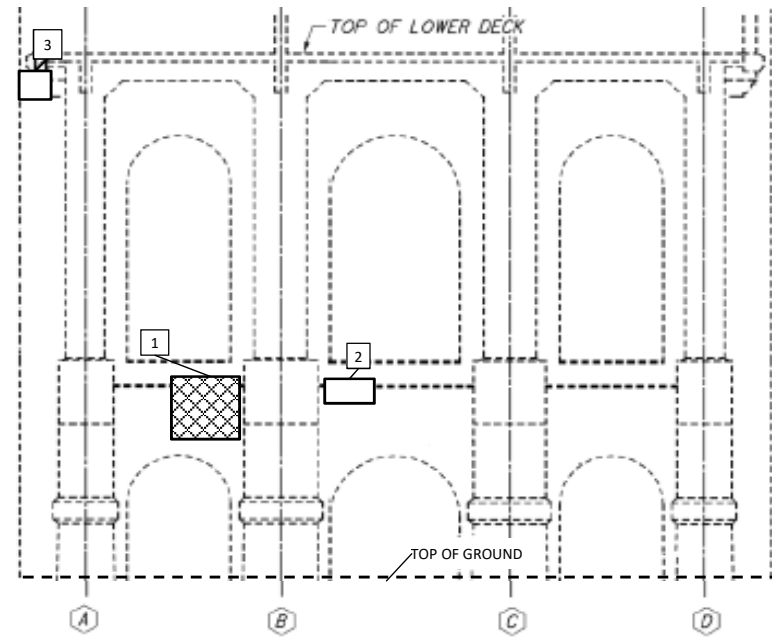








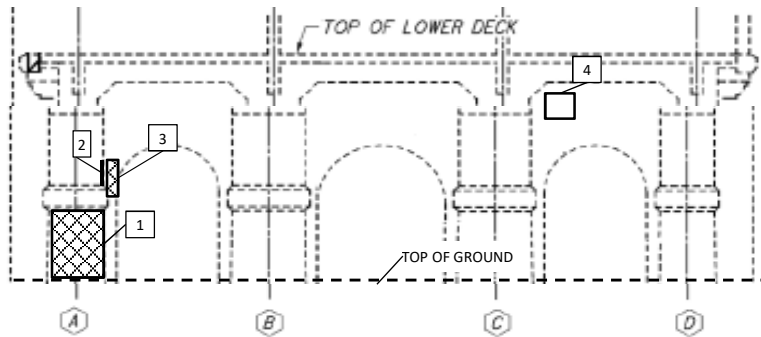
PIER 10 - WEST ELEVATION



PIER 10 - EAST ELEVATION

Pier 10 - West Elevation Deficiencies	
#	Deficiency
1	Delamination - 10 SF
2	Delamination - 21 SF
3	Spall - 5 SF

Pier 10 - East Elevation Deficiencies	
#	Deficiency
1	Map Cracking - 30 SF
2	Delamination - 3 SF
3	Spall - 2 SF



PIER 11 - WEST ELEVATION

Pier 11 - West Elevation Deficiencies	
#	Deficiency
1	Map Cracking - 30 SF
2	Map Cracking - 6 SF
3	Delamination and Map Cracking - 10 SF
4	Delamination - 2 SF

APPENDIX D – ASSETWISE BRIDGE INSPECTION FIELD REPORT

Ohio Bridge Inspection Summary Report

CUY-00006-1456 (1800930)

2: District 16000 - CLEVELAND (CUY county)
 ict
 12
 21: Major Maint A/B 01 - State Highway Agency /
 225 Routine Main A/B 04 - City or Municipal Highway /
 Agency
 221 Inspection A/B 01 - State Highway Agency /01
 220: Inv. Location DISTRICT 12

5A: Inventory Route 1 00006
 7: Facility On USR 6
 6: Feature Ints CUY. RIVER & RTA
 9: Location DETROIT/SUPERIOR BRIDGE
 Lat, Lon 41.49193438201303 , -81.70618648755347

Condition	Structure Type
-----------	----------------

58: Deck	6 - Satisfactory Condition	43: Bridge Type 3 - Steel
58.01 Wearing Surface	8 - Very Good (isolated or minor problems)	12 - Arch - Thru
58.02 Joint	6- Satisfactory (isolated leaking)	N- Not Applicable
59: Superstructure	5 - Fair Condition	45: Spans Main / Approach 1 / 12
59.01 Paint & PCS	6 - Satisfactory (5-10% corr.)	107: Deck Type 1 - Concrete Cast-in-Place
60: Substructure	6 - Satisfactory Condition	408: Composite Deck Y - Composite Construction
61: Channel	6	414A Joint Type 1 8 - Elastomeric Strip Seal
61.01 Scour	7 - Good	414B: Joint Type 2 3 - Compression Seal
62: Culverts	N - Not Applicable	108A: Wearing Surface 1 - Monolithic Concrete (concurrently placed with structural deck)
		N- Not Applicable
67.01 GA	5	

Appraisal

Sufficiency Rating 65.6 SD/FO 2 - FO
 36: Rail, Tr, Gd, Term Std 1 N 1 0
 72: Approach Alignment 8 - Equal to present desirable criteria
 113: Scour Critical 8 - Stable for scour conditions
 71: Waterway Adequacy 8 - Bridge Above Approaches

422: WS Date 11/15/1996
 423: WS Thick (in) 1.0
 482: Protective Coating 5 - Paint System OZEU
 483: PCS Date 07/15/1997
 453: Bearing Type 1 8 - Fixed Arch-Rib
 455: Bearing Type 2 3 - Sliding (Bronze)
 528: Foundn: Abut Fwd 4 - Spread Footing (on soil)
 533: Foundn: Abut Rear 4 - Spread Footing (on Soil)
 536: Foundn: Pier 1 N - None (Such as most Culverts)
 539: Foundn: Pier 2 N - None (Such as most Culverts)

Geometric

48: Max Span Length (ft) 591.0
 49: Structure Length (ft) 2656.0
 52: Deck Width, Out-To-Out (ft) 85.2
 424: Deck Area (sf) 226291.2
 32: Appr Roadway Width (ft) 84.0
 51: Road Width, Curb-Curb (ft) 72.0
 50A: Curb/SW Width: Left (ft) 5
 50A: Curb/SW Width: Right (ft) 5
 34: Skew (deg) 0
 33: Bridge Median 3 - Closed median with non-mountable barriers
 54B: Min Vert Underclearance (ft) 30
 336A: Min Vert Clrnce IR Cardinal (ft) 14
 336B: Min V Clr IR Non-Cardinal (ft) 0
 578: Culvert Length (ft) 0

Age and Service

27: Year Built/ 106 Rehab 1917 / 1997
 42A: Service On 5 - Highway-pedestrian
 42B: Service Under 7 - Railroad - waterway
 28A: Lanes on 06
 28B: Lanes Under 00
 19: Bypass Length 2
 29: ADT 20094
 109: % Trucks (%) 1

Load Posting

41: Op/Post/Closed A - Open
 70: Posting 5 - Equal to or above legal loads
 70.01: Date
 70.02: Sign Type
 734: Percent Legal (%) 110
 704: Analysis Date 12/09/2019
 63: Analysis Method 6 - Load Factor (LF) rating reported by rating factor (RF) method using MS18 loading.

Inspections

	<i>Months</i>	
90: Routine Insp.	12	10/29/2021
92A: FCM Insp. Y	24	10/05/2020
92B: Dive Insp. Y	60	07/08/2020
92C: Special Insp. N	0	
92D: UBIT Insp. N	0	12/23/2016
92E: Drone Insp.		
Inspector		Rufener, Justin

Inspector: Justin Rufener
 Inspection Date: 10/29/2021

Structure Number: 1800930
 Facility Carried: USR 6

Bridge Inspection Report

Element Inspection

	Environment	Total Quantity	Units	Condition State 1	Condition State 2	Condition State 3	Condition State 4
12 - Reinforced Concrete Deck	3 - Mod.	332900	sq. ft.	306576	24024	2300	0
	CS2: Areas of minor cracking with efflorescence, patching, delamination and saturation						
	CS3: Areas of spalling with and without exposed reinforcing.						
510 - Wearing Surfaces		246755	sq. ft.	241280	5475	0	0
	CS2: Areas of map cracking. Moderate transverse cracks in asphalt wearing surface.						
110 - Reinforced Concrete Open Girder/Beam	3 - Mod.	7394	ft.	6244	1000	150	0
	CS2: Areas of delamination and efflorescence.						
	CS3: Areas of spalling with exposed reinforcement.						
113 - Steel Stringer	3 - Mod.	10638	ft.	10338	200	100	0
	CS2: Areas of active corrosion.						
	CS3: Areas of active and painted over minor pitting. Isolated moderate section loss with corrosion holes.						
515 - Steel Protective Coating		101200	sq. ft.	95640	5060	500	0
	CS2: Areas of surface dulling and loss of effectiveness (substantially effective)						
	CS3: Isolated areas of limited effectiveness						
120 - Steel Truss	3 - Mod.	1182	ft.	490	500	192	0
	CS2: Areas of active surface corrosion						
	CS3: Areas of active and painted over pitting, pack rust, and rivet head loss.						
515 - Steel Protective Coating		60700	sq. ft.	44930	6070	9100	600
	CS2: Areas of surface dulling, loss of effectiveness (substantially effective), and peeling topcoat						
	CS3: Areas of limited effectiveness and peeling paint.						
144 - Reinforced Concrete Arch	3 - Mod.	8040	ft.	6740	1200	100	0
	CS2: Areas of minor to moderate cracking, poor patching and delamination on Columns and Arch						
	CS3: Areas of spalling with and without exposed reinforcing on Columns and Arch						
152 - Steel Floor Beam	3 - Mod.	3925	ft.	3575	200	150	0
	CS2: Areas of active surface corrosion						
	CS3: Areas of active and painted over pitting, and painted over perforations at deck openings.						

Inspector: Justin Rufener
 Inspection Date: 10/29/2021

Structure Number: 1800930
 Facility Carried: USR 6

Bridge Inspection Report

Element Inspection

515 - Steel Protective Coating		52950	sq. ft.	49770	2650	530	0
	CS2: Areas of surface dulling and loss of effectiveness (substantially effective) CS3: Isolated areas of limited effectiveness						
155 - Reinforced Concrete Floor Beam	3 - Mod.	33543	ft.	22543	10000	1000	0
	CS2: Areas of minor to moderate cracking, poor patching and delamination CS3: Areas of spalling with and without exposed reinforcing						
161 - Steel Pin and Pin & Hanger Assembly or both	3 - Mod.	30	each	27	3	0	0
	CS2: Minor painted over pitting on several eyebars below upper deck.						
162 - Steel Gusset Plate	3 - Mod.	100	each	20	47	33	0
	CS2: Areas of active surface corrosion CS3: Areas of active and painted over pitting and rivet head loss						
205 - Reinforced Concrete Column	3 - Mod.	513	each	340	100	73	0
	CS2: Locations of map cracking, failing patches and delaminations. CS3: Location of spalling with and without exposed reinforcing						
210 - Reinforced Concrete Pier Wall	3 - Mod.	200	ft.	100	50	50	0
	CS2: Areas of map cracking and delamination CS3: Deep spalls at base of Pier 3. Interior faces of Piers 3 & 4 have areas of spalling with exposed reinforcing.						
215 - Reinforced Concrete Abutment	3 - Mod.	3459	ft.	3159	200	100	0
	CS2: Areas of cracking with staining and delamination CS3: Areas of spalling						
300 - Strip Seal Expansion Joint	3 - Mod.	2579	ft.	1479	1000	100	0
	CS2: Areas of minor leakage and debris impaction. CS3: Areas of active leakage and broken welds in joint armor.						
313 - Fixed Bearing	3 - Mod.	4	each	0	4	0	0
	CS3: Pack rust around pins.						
330 - Metal Bridge Railing	3 - Mod.	1366	ft.	1366	0	0	0
331 - Reinforced Concrete Bridge Railing	3 - Mod.	5312	ft.	5208	100	4	0
	CS2: Areas of minor cracking with staining. CS3: Isolated spalling						
815 - Drainage	3 - Mod.	28	each	20	4	2	2
	CS2: Several partially plugged scuppers. CS3: Several plugged downspouts CS4: Several fully plugged scupper inlets						

Inspector: Justin Rufener
Inspection Date: 10/29/2021

Structure Number: 1800930
Facility Carried: USR 6

Bridge Inspection Report

Element Inspection

830 - Abutment Backwall	3 - Mod.	263	ft.	263	0	0	0
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ODOT District: District 12

CUY-00006-1456_(1800930)

Major Maint: 01 - State Highway Agency

Facility Carried: USR 6

Traffic On: 5 - Highway-pedestrian

Date Built: 07/01/1917

Rehab Date: 01/01/1997

Routine Maint: 04 - City or Municipal Highway Agency

Feature Inters: CUY. RIVER & RTA

Traffic Under: 7 - Railroad - waterway

Insp. Resp A: 01 - State Highway Agency

FIPS Code: 16000 - CLEVELAND (CUY county)

Location: DISTRICT 12

DETROIT/SUPERIOR BRIDGE

Insp Resp B: 01 - State Highway Agency

Inspector

Rufener,Justin

Inspection Date 10/29/2021

Reviewer Rufener,Justin

Inspector Comments - Deck and Approach

Deck

Element

12 – Reinforced Concrete Deck (SF)

The reinforced concrete deck is in Satisfactory condition.
The deck is divided into several sections as detailed below:

Detroit Avenue Tunnel: During the 1995-1997 rehabilitation a new reinforced concrete slab was placed on top of the original slab. The new slab was designed to support live and dead loads, with the original slab offering no structural support. The top and bottom surfaces for the new slab is not visible and assumed to be in good condition despite the poor and critical conditions of the original tunnel slab beneath.

West 25th Street Tunnel: The West 25th Street tunnel ceiling is in satisfactory condition, with areas of saturation, isolated delaminated areas and some shallow spalling with exposed reinforcing.

West Station: The West Station ceiling is in fair condition and has areas of spalling, cracking and efflorescence, active water infiltration, and exposed reinforcing steel.

Spans

1A, 1B, and 1 through 13: The upper deck floor in the main spans is in satisfactory condition. There are isolated cracks with some efflorescence, sound and unsound patches and spalls, some with exposed reinforcing. There are numerous areas of moisture staining, some of which have mottling.

East

Station: The East Station ceiling is overall in good condition with scattered cracking with efflorescence.

Lower Deck: The lower deck floor is not open to vehicular or pedestrian traffic is therefore not included as part of the element quantities. The lower deck floor is in good condition and consists of reinforced concrete with metal stay-in-place forms in Spans 1 through 3, and Spans 5 through 13. In isolated locations, the stay-in-place forms have active corrosion. In Span 4, the lower deck is an open steel grid type in middle section, and fiberglass grid in the exterior sections.

See the inspection report for additional details.

Element 300 – Strip Seal Expansion Joint

(LF)

The expansion joints are overall in Satisfactory condition. Joints typically have sections with loose debris and edge spalls along the joint armor. There are some areas of tearing in the joint seals accompanied by active leakage, and minor damage to joint armor. See the inspection report for additional details.

Element 330 – Metal Bridge Railing (LF)

The median railings

are in **Good** condition. The median railings are located along the edges of the roadway in Span 4 to protect the truss and hangers from vehicle impact.

Element 331 – Reinforced Concrete Bridge Railing (LF)

The concrete railings are in Good condition. The railings on the north and south side of the bridge consist of a reinforced concrete railing with an aluminum fence on top. All concrete railing is in good condition with minor cracking, staining, and isolated spalling. The fence has isolated areas of minor damage. See the inspection report for additional details.

Element 510 – Wearing Surface (SF)

The wearing surface is in Good condition. In Spans 1A – 13 and the East Station the wearing surface is a micro silica modified concrete, which was placed in 2019. There are isolated locations of map cracking in the concrete wearing surface. Above the Detroit Avenue Tunnel, West25th Street Tunnel and West Station, the wearing surface is asphalt. The asphalt has areas of transverse and map cracking. See the inspection report for additional details.

Element 815 – Drainage (EA)

The deck drainage is in Fair condition. The West Abutment south downspout and Pier 1 south downspout are completely clogged at the base of the catch basin. At Pier 3 the downspout is disconnected at the base of the pier, allowing drainage on the pier face. The Pier 9, South Catch Basin concrete frame has shifted and the north catch basin cover has shifted and rotated. The north sidewalk longitudinal trench drains are filled with debris and not functioning. Some of the scupper inlets are fully or partially clogged. At Pier 5 on the North Side, and in Span 13 on the South Side, there is active leakage coming through the utility entrances in the deck, allowing roadway drainage onto the maintenance deck. See the inspection report for additional details.

Curb/Sidewalk

The concrete curb and sidewalk are in Satisfactory condition. The curbs and sidewalks have areas of cracking, delamination, and spalling. The steel curb plates have widespread surface corrosion. See the inspection report for additional details.

Lighting

The lighting on the bridge is in **Fair** condition. Architectural light pole bases on the north sidewalk in Spans 5, 8 and 11 have cracked and are broken. Two of the architectural lights on the north sidewalk, and numerous of the taller, cobra style roadway lights are not functioning. All of the exterior pier shaft light brackets have paint failure and corrosion with minor section loss present. Many of the architectural lights attached to the lower deck fascia are not functioning, and several are visually broken.

Signs

The signs on the structure are in Good condition.

Approach

Approach Wearing Surface

The approach wearing surfaces are in Satisfactory condition. There are some areas of transverse and map cracking.

Embankment

The approach embankments are in Fair condition. The embankment under Spans 1 through 3 has several slope depressions. This embankment was primarily loose soil placed over demolition debris. Beneath this fill is are two concrete struts between Pier 2 and 3 used to maintain stability during construction. The south strut is preventing portions of the fill from sliding into the Cuyahoga River. This embankment is being monitored with slope inclinometers maintained by ODOT District 12. See the inspection report for additional details.

The embankment along the south side of Spans 1A and 1B has significant erosion for the full length. At the west end of the erosion, there is a 15' diameter x 4' deep erosion ditch around a manhole. An erosion ditch extends from the manhole towards the east typically 3' W x 2' D. This erosion is relatively unchanged from the 2020 inspection. Tower B South, which is in this area, is leaning due to slope instability, as previously discussed. See the inspection report for additional details.

Guardrail

The approach guardrails are in Good condition with some minor impact scrapes in the concrete rail.

Security Items

There are locations where the structure and structure right-of-way can be accessed by non-bridge personnel. The fence which encloses the area under Span 1 and along the south sides of Spans 1A and 1B is accessible due to an unlocked gate on the southeast end of Pier 1, and two locations where holes have been cut into the fence on the south side of Spans 1A & 1B. Due to these openings, there are multiple homeless encampments within the fenced in area. Preventative access steel mesh installed outside Span 1A near Tower A to prevent access has failed. Security fencing installed around Piers 2 and 3 can easily be surpassed, and there is evidence of a homeless encampment inside of Pier 3.

A chain link enclosure for the Center Street Bridge operator's vehicle on the west side of Pier 4 allows vandals to climb the fencing cover to access the Span 4 truss. From here the vandals have vandalized Pier 4 and have access to the truss lower chord and potentially the lower deck.

Inspector Comments - General Appraisal

Superstructure

Element 110 - Reinforced Concrete Beam (LF)

The beams are in overall Fair condition. This element consists of the longitudinal beams in the Detroit Avenue Tunnel, West 25th Street Tunnel, and West Station. The concrete beams have delaminations, efflorescence, and some areas of spalling with exposed reinforcing. See the inspection report for additional details.

Element 144 – Reinforced Concrete Arch (LF)

The concrete arches are in Fair condition. This element encompasses the concrete arches and arch columns. The concrete arches in Spans 5 through 10, Span 13 and portions of Span 3 were patched, crack injected and then wrapped on the underside with FRP to prevent future spalling. Select columns were also

patched. The concrete arches and columns that were not repaired typically have areas of cracking, delamination, poor patching and spalling with and without exposed reinforcing. The concrete jack arches connecting the columns below the upper and maintenance decks have spalls with exposed reinforcing steel, cracks, and delaminated areas.

See the inspection report for additional details.

Element 155 – Reinforced Concrete Floor Beam (LF)

The concrete floorbeams in Spans 1A, 1B, 1 through 3, and Spans 5 through 13 are in Satisfactory condition. The floorbeams have isolated spalls with and without exposed reinforcing, cracking, delaminations, and areas of poor patching. The lower deck floorbeams tend to be in worse condition than the upper deck floorbeams. The structural corbels are included in the rating of this element exhibit similar defects as the rest of the floorbeams. The lower deck floorbeams in the East Station have the bottom mat of reinforcing steel exposed. This deterioration has changed little since the 1980s, but they carry no substantial live load. As part of the most recent rehabilitation, the undersides of lower deck floorbeams in Spans 5 through 10, 13 and portions of Span 3 were fiber wrapped after patching, to prevent future spalling.

See the inspection report for additional details.

Element 113 – Steel Stringer (LF)

The stringers are in Satisfactory condition. There are 18 lines of stringers in the upper deck and 12 in the lower deck. The upper deck stringers have shear studs welded to the top flange providing composite action with the deck. The upper and lower deck stringers in Panels 4, 5, 5', and 4' were replaced in 1995. The upper deck stringers are in good condition. The original curb stringers of Lines E and N have areas of painted over pitting, with some active corrosion.

The lower deck is not open to vehicular or pedestrian traffic the stringers are therefore not included as part of the element quantities. The stringers supporting the steel grid deck are in good condition. Stringers D, E, I and J, which support only their own dead load, often have painted over advanced section loss and perforations at the floorbeams and saddle bearings. Stinger K, which supports the south fiberglass pedestrian deck has a similar locations of advanced section loss. The rest of the stringers supporting the outer pedestrian fiberglass grid deck are in good condition.

See the inspection report for additional details.

Element 120 – Steel Truss (LF)

The steel truss is overall in **Satisfactory** condition. There are areas of pack rust, pitting and surface corrosion, mainly at and below the upper deck. Perforations, many of which have been cleaned and painted over are present in the diaphragm plates and lacing bars. At the eyebar connections there are areas of painted over pitting in the web plates.

See the inspection report for additional details.

Element 152 – Steel Floor Beam (LF)

The steel floorbeams are in **Satisfactory** condition. The floorbeams typically have painted over perforations near the deck openings at the truss lines, with repair plates welded in place at some of these locations. Active surface corrosion is present at due to ongoing water infiltration at the deck openings.

On lower deck floorbeams 10, 12 and 11' through 6' there are cracks along the weld of the stiffening plates to the top flange at the north truss line. Some of the crack lengths have changed in length from the 2020 Inspection. The cracks at Floorbeam 12 were not noted in previous inspections.

See the inspection report for additional details.

Element 161 – Steel Pin & Hanger Assembly (EA)

The pins, hangers and hinges are in **Good** condition with no significant deficiencies noted. Minor painted over pitting was noted on some eye-bars below the upper deck. There is active corrosion on some of the hangers above the previous zonal painting. See the inspection report for additional details.

Element 162 – Steel Gusset Plates (EA)

The truss gusset plates are in **Fair** condition. The gusset plates typically have areas of active surface corrosion. The lower chord gusset plates below the upper deck typically have painted over pitting and reactivating corrosion along the top of the lower chord. At North Truss L2, the north gusset plate has 2' L x 3" H x up to 1/8" D reactivating pitting at the lower chord interface on the south face and 2' L x up to 3" H x up to 5/16" D pitting on the north face. The south gusset plate has 30" L x up to 4" H x up to 3/8" D pitting on the south face with reactivating corrosion and 2' L x up to 3" H x up to 1/16" D pitting on the north face. At the North Truss L3, the gusset plates have pitting up to 3/16" D. In other scattered locations, the gusset plates have areas of pitting. See the inspection report for additional details.

Lateral Bracing & Sway Bracing

The lateral bracing and sway bracing is in **Satisfactory** condition with isolated areas of active surface corrosion, pack rust, and advanced section loss including perforations. See the inspection report for additional details.

Element 313 – Fixed Bearing (EA)

The bearings are in Fair condition with some pack rust around the pins, and surface corrosion noted on the interior faces of all four bearing castings. The non-structural bearing pin cover plates have cracks up to 7 inches long at L0 and L0' on both trusses. The north pin cover at L0 on the north truss has fallen off. There is advanced section loss of some of the anchor bolt and nuts

Between the deck underside and the top of the transverse floorbeams over Piers 11 & 12, there are 3" H concrete pedestals with galvanized steel plates sitting on top and between each pedestal. In several locations these plates have moved and in some cases are no longer support the

deck underside. See the inspection report for additional details.

Item 515 – Steel Protective Coating (SF)

The protective coating system (PCS) is in **Satisfactory** condition. Areas of corrosion, peeling and failed paint are present on the main truss members below the lower deck. The structural steel between the upper and lower decks was repainted in 2014-2015 and is in good condition. The protective coating system above the upper deck has areas of fading and surface corrosion with minor rust staining.

Fatigue Prone Details

The fatigue prone details are in **Fair** condition. Stiffening retrofit plates welded to the top flange of the lower deck floorbeams at the truss lines are classified as Category E fatigue details. Cracks in the fillet welds are present at several locations. Refer to *Element 152 – Steel Floor Beam* above for additional details on crack locations and growth.

Utilities

The utilities are in Satisfactory condition. The lower deck telephone junction chambers and supports are corroded due to saltwater infiltration through the manhole above. At Pier 5 on the North Side, and in Span 13 on the South Side, there is active leakage coming through the utility entrances in the deck. A utility line mounted below the top deck in the south bay is sagging and making contact with the maintenance deck in several locations. There is a missing electric junction box cover on the south concrete rail in Span 7.

Substructure

Element 205 – Reinforced Concrete Column (EA)

The pier columns are in Satisfactory condition. This item includes the main span pier columns, columns in Spans 1A and 1B, and the columns in the subway tunnels and stations. The main span pier columns have areas of map cracking, failing patching, and delamination. There are also a few areas of minor spalling with exposed reinforcing. The decorative arches above these columns have areas of map cracking, failing patching, delamination and spalling with exposed reinforcing. The remaining columns have areas of failing patching, delamination, and spalling with and without exposed reinforcing.

The decorative towers on the north and south faces of the piers typically have widespread cracking, delamination and spalling with and without exposed reinforcing. The south towers at Piers 5 through 7 are leaning away from bridge, with gaps up to 2¼-in between the top of the tower and the outside face of the upper jack arches. Detailed measurements of the gaps between these towers and the adjacent bridge features are given Table 5 in Appendix B. No change was noted between the current measurements and those taken during the 2020 inspection.

See the inspection report for additional details.

Element 210 – Reinforced Concrete Pier Wall (LF)

The pier walls at Piers 1, 3 and 4 are in Satisfactory condition. The west face of Pier 1 is primarily covered by fill. The

exposed portions of the pier walls have areas of map cracking and delamination. Piers 3 and 4 are located adjacent to the Cuyahoga River. The portion of Pier 3 that is exposed to the channel has widespread areas of deep abrasion and numerous spalls. Piers 3 and 4 are cellular type structures, which are open on their Span 3 and 5 faces, respectively. The interiors faces of the walls have areas of delamination and spalling with exposed reinforcing. See the inspection report for additional details.

Element 215 – Reinforced Concrete Abutment (LF)

The abutment walls are in Satisfactory condition. The abutment walls consist of the West and East Abutments and the walls of the Detroit Avenue and West 25th Street Tunnels. The abutments have areas of cracking with minor moisture staining, delamination and some spalling. Some staining appears to be superficial due to leaking deck joints above. In the tunnels, the lower 1-ft to 2-ft of the walls have widespread shallow spalling with exposed reinforcing. The portions of the walls in the tunnels above these areas were repaired as part of the most recent rehabilitation. . See the inspection report for additional details.

Element

830 – Abutment Backwall (LF)

The backwalls are in Good condition. The backwalls consist of the closure panels at the ends of the West 25th Street Tunnel, Detroit Avenue Tunnel, and East Station.

Wingwalls

The wingwalls are in Poor condition. The wingwalls along Spans 1A and 1B and the East Station have cracking and spalling with exposed reinforcement throughout. See the inspection report for additional details.

Tower B South

A section of the rear abutment, south wall at Tower B has through cracks in the wall and associated footing, and is leaning to the south. It has continued to show incremental movement over the past 10+ years. On the interior, the top of the tower is spalled and cracked due to contact with the soffit of the upper level sidewalk. Crack gages have been placed at several locations to monitor the movement of the section. Crack gauges located at the base of Tower B are cracked and slightly displaced. New gauges should be installed to ensure an accurate record of the tower rotation is maintained. See the inspection report for additional details.

West and East Abutment Chambers

The chambers below Spans 1A and 1B on the west approach and below the East Station were inspected, however, they are not included in any of the quantities within this report. There are large spalls and delaminations throughout the chambers with exposed and corroded reinforcing on the walls and ceilings of most of the cells. Horizontal, vertical, diagonal and map cracking with efflorescence and moisture staining are also present throughout all cells. The floors are typically covered in dirt and construction debris. In the west chamber this is heavy cracking around south Tower B (see discussion above for more details).

Water Infiltration

Standing water was noted in the pedestrian tunnel under the West Station. In the east abutment chamber, most of the lower cells are filled with standing water. The pedestrian tunnel under the East Station is also filled with standing water. Holes drilled in the east abutment to drain some of the standing water had a steady flow of water at the time of inspection.

Slope

Protection

The slope protection is in Satisfactory condition, with some areas of erosion and sliding material noted.

Culvert

Inspector Comments - Waterway

Waterway Adequacy

Channel

Alignment

The alignment is in Good condition. The channel is skewed with respect to the piers, but this is an as-built condition.

Protection

The channel protection is in Satisfactory condition with only minor deficiencies. The west bank is vegetated with some dumped rock channel protection. The west bank is protected by a sheet pile wall.

Hydraulic Openings

The hydraulic opening is in Good condition with no major constrictions associated with the bridge.

Navigation Lights

The navigation lights are in Poor condition. None of the six lights were functioning at the time of inspection. No damage was noted to the light fixtures.

Scour

The scour is in Good condition. An underwater bridge inspection was performed on July 8, 2020. No areas of exposed foundation or significant scour holes were found in the inspection. See the underwater inspection report for additional details.

Scour Critical