

**REPORT
OF
SUBSURFACE INVESTIGATION
FOR
EMBANKMENTS (STATION 416+00 to 509+50)
PROJECT SCI-823-6.81
PHASE 1 – STAGE I
SCIOTO COUNTY, OHIO**

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1.0 INTRODUCTION

Phase 1 of this project consists of the construction of approximately 3.51 miles of new interstate roadway situated in Scioto County, Ohio. The findings and recommendations presented in this document pertain to Phase 1 embankments from station 416+00 to 509+50 of the SCI-823-6.81, Portsmouth Bypass project. The Phase 1 area is understood to be from station 352+00 to 537+50. The findings and recommendations for Phases 2 and 3 of this project will be submitted in separate documents.

It should be noted that two interchanges are located in the Phase 1 area. The relocated Shumway Hollow Road interchange is located from approximately station 352+00 to 416+00. Similarly, the Lucasville-Minford Road interchange is located from approximately station 509+50 to 538+55. The analyses and recommendations pertaining to the interchange areas can be found in the respective interchange reports, submitted separately.

The purpose of this exploration was to 1) determine the subsurface conditions to the depths of the borings, 2) evaluate the engineering characteristics of the subsurface materials, and 3) provide information to assist in designing the rock cut slopes, roadway embankments and pavements.

The geotechnical engineer has planned and supervised the performance of the geotechnical engineering services, has considered the findings, and has prepared this report in accordance with generally accepted geotechnical engineering practices. No other warranties, either expressed or implied, are made as to the professional advice included in this report.

2.0 GENERAL PROJECT INFORMATION

Approximately 2.35 miles of side hill or embankment fills and 1.16 miles of soil and rock cuts are proposed to construct the mainline roadway in the Phase 1 area. Throughout the Phase 1 area, a maximum embankment height is anticipated to be 73 feet near station 466+50. Similarly, in the Phase 1 area, the maximum height of the soil and rock cuts is anticipated to be 132 feet at station 428+80.

Two proposed interchanges, relocated Shumway Hollow Road (TR 234) and Lucasville-Minford Road (CR 28) are also included in the Phase 1 area. As mentioned above, the findings and recommendations pertaining to the roadway embankments and ramps included in the interchanges

are presented under separate cover. Four structures are planned for the Phase 1 mainline alignment of this project. Two structures are proposed, one for each of the above referenced interchanges. Additionally, structures are proposed for crossing Portsmouth-Minford Road (SR 139) and Swauger Valley-Minford Road at stations 485+07 and 443+85, respectively. The findings and recommendations pertaining to the structures are presented under separate cover.

The analyses and recommendations presented in this report have been made on the basis of the foregoing information. If the proposed roadway location or concept is changed or differs from that assumed, DLZ should be informed of the changes so that recommendations and conclusions presented in this report may be revised as necessary.

3.0 SUBSURFACE INVESTIGATION

The subsurface investigation for the Phase 1 embankments (stations 416+00 to 509+50) consisted of drilling a total of 83 borings. The 83 borings considered for these roadway embankment analyses are comprised of 17 structural borings, 10 culvert borings, and 56 roadway borings. The borings were drilled between July 9, 2004 and September 14, 2006. The borings were extended to depths of 8.0 to 173.0 feet and were drilled with both ATV-mounted and truck-mounted drill rigs. The locations of the borings are shown on the Boring Location Plan in Appendix A.

Representatives of DLZ Ohio, Inc. (DLZ) planned and staked the interchange boring locations in the field. Similarly, representatives of Lockwood, Lanier, Mathias and Noland Inc (2LMN) staked the locations of the non-interchange roadway borings in the Phase 1 area. Representatives of 2LMN also determined the as-drilled ground surface elevations and locations of most of the boring locations. The as-drilled boring locations and ground surface elevations are shown on the individual boring logs in Appendix A. At the time this document was drafted, the as-drilled locations of several boring locations had not been established. In lieu of survey information, the as-per-plan stations, offsets, and elevations were estimated and are included on the boring logs for boring locations that were not surveyed. Information concerning the drilling procedures and the boring log terminology is also presented in Appendix A.

4.0 FINDINGS

4.1 General Information

The project is located in the Shawnee-Mississippian Plateau of the unglaciated portion of the Appalachian Plateau Physiographic Region. The project area is relatively underdeveloped, and contains limited secondary roadways. The area is characterized by rough, steep, broken, and severely dissected topography. The natural slopes are generally very steep, rising abruptly from the valley bottoms. The maximum topographic relief along the Phase 1 project centerline is approximately 275 feet and occurs between a high point near station 523+70 at elevation 890 feet, and a low point near station 353+80, at approximately elevation 615 feet. The maximum vertical relief along the proposed finished grade is approximately 130 feet,

with the lowest point at approximately station 352+00 (elevation 645 feet) and the highest point near station 519+30 (elevation 775 feet).

4.2 Geology of the Site

The genesis of the soils varies across the project location. Residual and colluvial soils are found on the ridge tops and the hillsides across the site. These soils are generally thin to moderately deep, covering moderate to steep slopes. Lacustrine soils, found in the valleys, are commonly known as “Minford Silts” or the Minford Complex. These deposits were formed during the early to middle Pleistocene age when the northward flowing Teays River system was blocked by the southward advance of the Kansan aged ice sheets. As the glaciers advanced, the course of the Teays River was blocked south of Chillicothe and a large lake was formed from the impoundment of the waterways. As a result of the impoundment, vast quantities of sediments were deposited ranging from 10 to 80 feet in thickness, thinning towards the margins. In this area, the Minford Complex is characterized by clays of high plasticity and high compressibility.

Bedrock within the structure area is primarily sandstone of the Logan Formation that is of Mississippian Age. Bedrock of the Pennsylvanian Breathitt Formation can be found at the top of the slopes, roughly above elevation 870.

4.3 Soil Conditions

At the surface, 65 of 83 borings encountered topsoil ranging in thickness from 1 to 12 inches. Where encountered, the average thickness of topsoil was 5 inches.

Beneath the topsoil layers, borings located on the slopes and drilled for the roadway generally encountered soils ranging from silt (A-4b) to clay (A-7-6) to depths of 5 to 50 feet below the ground surface, at the top of bedrock. Soil conditions for selected Phase 1 fill sections are outlined below. For more detailed information, refer to the boring logs presented in Appendix A.

Phase 1 Mainline Station 434+00 to 449+00

In this area, boring R-379, at station 440+12.8 generally encountered hard silt (A-4b) to a depth of 11.0 feet below the ground surface. Beneath this layer, very stiff sandy silt (A-4a) was encountered to a depth of 18.0 feet below the ground surface, at the top of bedrock.

Phase 1 Mainline Station 457+00 to 479+00

In this area, boring R-393, at station 460+18.5 generally encountered cohesive and cohesionless medium dense/very stiff sandy silt (A-4a) to a depth of 30.0 feet below the ground surface. Beneath this layer, very stiff silt (A-4b) was encountered to a depth of 38.5 feet below the ground surface, at the top of bedrock.

Phase 1 Mainline Station 483+50 to 497+50

In this area, boring B-10, at station 486+01.5 generally encountered stiff silt (A-4b) to a depth of 8.5 feet below the ground surface, at the top of bedrock.

Phase 1 Mainline Station 504+00 to 507+50

In this area, the soil is very thin. Boring R-439, at station 504+62 generally encountered loose to medium stiff sandy silt (A-4a) to a depth of 4.5 feet below the ground surface, at the top of bedrock.

4.4 Bedrock Conditions

Bedrock was encountered in many of the borings and confirmed by coring in 76 borings. Bedrock encountered in the borings correlates well with the available geologic references. The cores obtained consist primarily of sandstone and occasionally shale, siltstone, and clayshale. A layer of severely weathered to decomposed bedrock was generally encountered immediately above the higher quality rock encountered in the rock cores. The layer of severely weathered rock generally ranged in thickness from 1 to 3 feet. The bedrock encountered in the cores generally consisted of medium hard to hard, fine-grained sandstone, which is slightly to highly weathered. For more detailed information, refer to the boring logs presented in Appendix A.

4.5 Groundwater Conditions

Seepage was observed in several borings at depths ranging from 5 to 58.5 feet below the ground surface. Groundwater conditions for selected Phase 1 fill sections are outlined below. For more detailed information, refer to the boring logs presented in Appendix A.

Phase 1 Mainline Station 434+00 to 449+00

In this area, no seepage or groundwater levels were observed in any of the borings prior to adding drilling water for coring operations. Final water levels (including drill water) were measured from 3.5 to 17 feet below the ground surface.

Phase 1 Mainline Station 457+00 to 479+00

In this area, seepage was encountered in 9 borings. In these borings, seepage was first encountered ranging from 2.5 to 19 feet below the ground surface. Groundwater levels were observed in two borings prior to adding drilling water for coring operations. In both borings, groundwater was encountered at 20 feet below the ground surface. Final water levels (including drill water) were measured from 0.5 to 17.4 feet below the ground surface.

Phase 1 Mainline Station 483+50 to 497+50

In this area, seepage was encountered in 4 borings. In these borings, seepage was first encountered ranging from 3 to 6.3 feet below the ground surface. No groundwater levels were observed in any of the borings prior to adding drilling water

for coring operations. Final water levels (including drill water) were measured from 1.6 to 16.3 feet below the ground surface.

Phase 1 Mainline Station 504+00 to 507+50

In this area, no seepage or groundwater levels were observed in any of the borings prior to adding drilling water for coring operations. Final water levels (including drill water) were measured from 4.8 to 6.8 feet below the ground surface.

It should be noted that groundwater levels may fluctuate with seasonal variations and following periods of heavy or prolonged precipitation, and therefore, the readings indicated on the boring logs may not be representative of the long-term groundwater levels. Long-term monitoring would be needed to obtain a more accurate estimate of the groundwater table elevation.

4.6 Laboratory Testing

In the laboratory, all samples were examined and visually classified by a soils engineer. The moisture content, grain size analysis, and plasticity characteristics of samples considered representative of the subsurface materials were determined. Due to the consistency of the soils encountered in this area, undisturbed Shelby tube samples were not obtained for laboratory testing. The results of the laboratory testing are presented in Appendix B, and on the boring logs in Appendix A.

5.0 CONCLUSIONS AND RECOMMENDATIONS

5.1 General

At the surface, 65 of 83 borings encountered topsoil ranging in thickness from 1 to 12 inches. Where encountered, the average thickness of topsoil was 5 inches. All topsoil should be removed prior to placing fill or pavement materials.

Subgrades and embankments should be constructed in accordance with Ohio Department of Transportation Construction and Material Specification (ODOT-CMS) Item 203, "Roadway Excavation and Embankment."

5.2 Pavement Design and Group Index

5.2.1 General

The results of the borings and the subgrade evaluations for the project (Phase 1) have been submitted in separate documents. However, the conclusions from the proposed State Route 823 Mainline evaluations are presented below. It should be noted that unsuitable soils were encountered in the Phase 1 area. Restrictions for the use of these materials should conform to ODOT CMS Item 203.03. Similarly, all subgrade

treatments should conform to ODOT CMS Item 206. More information concerning the suitability of soils for use as fill material can be found in section 5.6 of this report.

5.2.2 Proposed State Route 823 Mainline

The method used to calculate the CBR value was essentially the same as that used by the Office of Geotechnical Engineering (OGE) to determine the design CBR value for the Nelsonville Bypass project. The alignment for that project was similar to the Portsmouth Bypass alignment in that most of it will also be in rock cuts or on embankments. In Phase 1, 242 samples were tested for particle size and plasticity.

Existing laboratory test results performed as of June 6, 2006 were evaluated to estimate a recommended CBR value for Phase 1 mainline roadway pavement design. For the proposed Phase 1 mainline alignment and the ramps, it was recommended that the pavements be designed based on a CBR value of 6.

Most of the mainline alignment will be constructed on embankment fill. However, some sections of the alignment will be in cuts where the subgrade will be in rock. The approximate station limits of these sections are listed in the following table.

Sections of Proposed State Route 823 Mainline Grade in Rock (Phase 1) Station 416+00 to 509+50

Begin Station	End Station
416+00	434+00
449+00	457+00
480+00	483+50
497+50	504+00
507+50	509+50

For the sections of the alignments with subgrades in rock, excavations 2 feet below the proposed pavement materials will be required for the subgrade preparation in accordance with ODOT CMS Item 204.05.

5.3 Culverts

As discussed earlier in this report, 21 culverts are presently planned for within the limits of the Phase 1 area. Foundation recommendations and settlement analyses for the culverts have been presented for each culvert in separate reports.

5.4 Embankment Evaluations

5.4.1 Slope/Embankment Stability – State Route 823 Mainline

With the exception of the two interchange areas (presented under separate cover), slope/embankment stability is not considered to be a significant concern for most areas of the proposed State Route 823 mainline alignment. The following table outlines the station locations and approximate embankment heights for the proposed Phase 1 mainline embankments.

Sidehill Fill / Fill Embankments (STA. 416+00 to 509+50)

Begin Station	End Station	Approximate Maximum Fill (ft.)
434+00	449+00	44.3
457+00	479+00	70.6
483+50	497+50	58.9
504+00	507+50	34.7

Soil parameters used for the stability and settlement analyses were based on laboratory test results (grain-size and plasticity), visual examination of the preserved samples, hand penetrometer readings, and typical values. Due to the consistency of the soils encountered in this area, undisturbed Shelby tube samples were not obtained for laboratory testing. Global stability analyses and settlement calculations are presented in Appendix C.

In accordance with ODOT guidelines a unit weight of 120 pcf was used for the embankment fill materials. Due to the nature of the project, it is anticipated that the embankment fill will consist of cohesionless material ranging in size from fine granular material to rock but will generally be rock fill from adjacent cuts. The friction angles of the anticipated backfill materials will likely range from 28 degrees to over 40 degrees. We would anticipate that more of the rock fill would exhibit friction angles in excess of 40 degrees, but we conservatively selected a friction angle of 35 degrees for the embankment fill with no cohesion.

The stability analyses were performed using UTEXAS3 Version 1.204, a slope stability computer program using variations of the method of slices. UTEXAS3 was developed by Dr. Stephen Wright at the University of Texas for the U.S. Army Corps of Engineers. The Simplified Bishop procedure was used for all of the analyses and only circular failure surfaces were considered. All of the procedures use an iterative approach to investigate many failure surfaces until a critical surface is found. The results of stability analyses are included in Appendix C.

5.4.1.1 Phase 1 Mainline Embankments – General Information

The Phase 1 mainline roadway is comprised of approximately eight fill sections and five cut sections. All fill sections in Phase 1 were considered for stability. The two interchange areas (Shumway Hollow Road and Lucasville-Minford Road Interchanges) contain the poorest subsurface conditions in the Phase 1 area. The analyses and recommendations for the embankment slopes contained in the interchange areas are found within the respective interchange reports, presented separately.

A comprehensive review of subsurface conditions and the results of stability analyses indicate that the Phase 1 embankments, outside of the interchange areas may be built using 2H:1V or flatter slopes. A brief discussion of the analyses is presented below.

5.4.1.2 Phase 1 Mainline Embankments (Station 416+00 to 509+50)

The Phase 1 alignment from station 416+00 to 509+50 contains four sections, which will be constructed using fill embankments. Within these four sections, the soil is generally thin. The subsurface profile encountered in boring R-393 is assumed the most critical in this area. Additionally, the maximum embankment height encountered in this area, 73 feet, was coupled with the subsurface profile encountered in boring R-393 to evaluate the stability of 2H:1V embankment slopes.

Assuming 2H:1V slopes, global stability analyses performed for the mainline embankments in this area have indicated that the drained, seismic and undrained stability of the embankments is adequate. Based upon the results of the stability analyses, it is not anticipated that staged construction, wick drains, or other special construction techniques will be required to construct the embankments using 2H:1V slopes in this area. As a result, it is recommended that roadway embankments be built using 2H:1V for flatter slopes.

5.4.2 Settlement – State Route 823 Mainline

5.4.2.1 Phase 1 Mainline Embankments (Station 416+00 to 509+50)

Settlement has been evaluated for selected locations for the Phase 1 mainline embankments. The table on the following page summarizes the results of the calculations for the consolidation of foundational soils and the anticipated settlement of the associated embankment fill materials.

Summary of Settlement Analyses

Station	Boring	Approximate Maximum Fill (ft.)	Primary Consolidation (in.)	Anticipated Settlement of Fill Materials (in.)
441+00	R-379	44.3	2	6
465+00	R-393	70.6	5	9
487+00	B-10	58.9	2	8

The settlement evaluations indicate that embankment settlement is not considered a significant concern for Phase 1 embankments outside of the interchange areas. Based upon a comprehensive review of subsurface conditions, a critical soil profile and embankment height was selected for each fill section for the purposes of performing settlement calculations. In addition, time-rate of settlement calculations were also performed. The analyses and results are briefly discussed in the following sections.

Settlement – Mainline Embankments, Station 441+00

Settlement due to primary consolidation of the foundation soil is expected to be approximately 2 inches. In addition, the time needed to reach 80 percent of primary consolidation is expected to take up to approximately 369 days. Due to the relatively small magnitude of settlement, the use of wick drains or other means of accelerating the consolidation were not explored.

Settlement due to consolidation of the fill material itself was also considered. Consolidation within an embankment will generally range from one to four percent of the embankment height. Assuming one percent consolidation for a well compacted fill, approximately 6 inches of settlement can be expected for a 44 foot high embankment. However, it is anticipated that much of this settlement will occur during construction.

Settlement – Mainline Embankments, Station 465+00

Settlement due to primary consolidation within the foundation soil is expected to be approximately 5 inches. In addition, the time needed to reach 80 percent of primary consolidation is expected to take up to approximately 1,060 days. Due to the relatively small magnitude of settlement, the use of wick drains or other means of accelerating the consolidation were not explored.

Settlement due to consolidation of the fill material itself was also considered. Consolidation within an embankment will generally range from one to four percent of the embankment height. Assuming one percent consolidation for a well compacted fill, approximately 9 inches of settlement can be expected for a 71 foot high embankment. However, it is anticipated that much of this settlement will occur during construction.

Settlement – Mainline Embankments, Station 487+00

Settlement due to primary consolidation within the foundation soil is expected to be approximately 2 inches. In addition, the time needed to reach 80 percent of primary consolidation is expected to take up to approximately 68 days. Due to the relatively small magnitude of settlement, the use of wick drains or other means of accelerating the consolidation were not explored.

Settlement due to consolidation of the fill material itself was also considered. Consolidation within an embankment will generally range from one to four percent of the embankment height. Assuming one percent consolidation for a well compacted fill, approximately 8 inches of settlement can be expected for a 59 foot high embankment. However, it is anticipated that much of this settlement will occur during construction.

Settlement –Mainline Embankments, Station 504+00 to 507+50

Due to the very thin soil in this area, settlement was not evaluated for this embankment fill area. It is believed that any settlements in this area will be negligible. Borings drilled in the area encountered 3 to 4.5 feet of soil throughout the embankment fill area. The maximum embankment height in this area was 35 feet.

Settlement due to consolidation of the fill material itself was considered. Consolidation within an embankment will generally range from one to four percent of the embankment height. Assuming one percent consolidation for a well compacted fill, approximately 4 inches of settlement can be expected for a 35 foot high embankment. However, it is anticipated that much of this settlement will occur during construction.

5.4.2.2 Additional Settlement Recommendations

Prior to beginning normal embankment fill operations, it is recommended that the initial embankment construction consist of at least 6 feet of free-draining granular material placed over the entire fill foundation area. This material will allow the drainage of the foundation soils, not inhibit the time-rate of consolidation, and will also provide a stable surface upon which normal fill operations can begin. It should be emphasized that the time of consolidation estimates are based on the assumption that the initial embankment construction will consist of at least 6 feet of free-draining granular material placed over the entire fill foundation area as previously discussed. Groundwater seepage, overexcavation, and removal of unsuitable soils should be anticipated within the interchange areas. Consequently, it may be necessary to utilize additional granular material or end-dumped rock in order to establish a dry and stable fill foundation.

Based on the results of calculations outlined above, excessive settlements are not anticipated for the Phase 1 embankments from station 416+00 to 509+50. Although settlement is not a significant concern in this area, general recommendations are included for accelerating or mitigating settlements.

The most cost effective method for dealing with the potentially excessive settlement would be to surcharge the embankment foundations prior to construction. Monitoring equipment should be installed to measure the rate/amount of settlement and normal fill operations should begin when an acceptable degree of consolidation is achieved. However, given the anticipated construction schedule, it will likely be necessary or desirable to either reduce the amount of settlement in these areas or to accelerate the time of consolidation within the embankment foundation. One option that exists to reduce the amount of settlement would be to consider additional overexcavation and replacement of the existing soft and/or organic soils. Alternately, wick drains could be considered in order to accelerate the time of consolidation of the embankment foundation.

Even if wick drains are to be considered, it is still recommended that the initial embankment construction consist of at least 6 feet of free-draining granular material placed over the entire fill foundation area. This will allow better dissipation of excess pore pressures that may occur and will also provide drainage for the outflow from the wick drains. In addition, the wick drain installation should be performed by a contractor who specializes in their installation.

5.4.3 Potential Areas of Instability

The dominant rock type along the proposed alignment is sandstone of the Mississippian aged Logan Formation. Siltstone and shale are commonly found interbedded with the sandstone. These siltstones and shales generally weather to clay with low shear strength over time. The steeper slopes are prone to gradual movement known as soil creep. The low shear strength of the residual and colluvial soils combined with the steep topography makes some of the hillsides within the proposed limits of construction prone to shallow surficial landslides and soil creep. Generally these conditions are easily corrected by removal of the unstable slope materials. No deep-seated landslides were observed along the proposed Phase 1 alignment. If the proposed alignment is in a fill section, it is recommended that these areas of instability be monitored during future design stages for any signs of slope movement.

In the steep terrain of Scioto County, soil creep is common. Areas of slope instability were first identified using survey data and aerial photography and then verified during the fieldwork. Four areas showed recent signs of significant instability near or

within the proposed limits of construction. All slope instability appeared to be relatively shallow soil creep and contained within the overburden, though one of these areas showed signs of a past massive landslide. These areas of slope instability are shown on the field notes and proposed centerline in Appendix A of the *Report of Geology and Field Reconnaissance for Project SCI-823-6.81, Phase 1-Stage I, dated November 29, 2006.*

Station 352+00 to Station 356+00

The aerial photography showed hummocky terrain from station 352+00 continuing to station 356+00. This area exhibited signs of recent instability. This is most likely due to erosion and shallow soil creep along the steep drainage channels of intermittent streams in the area.

Station 423+00 to Station 425+00

The area from station 423+00 to station 425+00 exhibited signs of recent instability. This is most likely due to erosion and saturation of the steep, nearly 1H:1V slopes along steep intermittent stream channels during heavy rainfall events. The instability in this area appeared to be relatively massive and was classified as a landslide. However, in this area, the current profile indicates the alignment will be in a deep rock cut section and the unstable soils will be removed.

Station 432+00 to Station 442+00

Another area to exhibit signs of instability is the steep slope from station 432+00 to station 442+00. This is most likely erosion from logging activities and the toes of these slopes being eroded by the intermittent streams that flow through the valley. The recent activity appears to be shallow soil creep and not a deep active landslide.

Station 482+00 to Station 484+25

Another area to exhibit signs of instability is the steep, nearly 1H: 1V slope from the station 482+00 to station 484+25. This is most likely due to the toe of the slope being eroded by Long Run. The recent activity appears to be shallow soil creep and not a deep active landslide.

5.5 Rock Cuts

Currently, approximately 1.16 miles of the Phase 1 mainline alignment is anticipated to be located in cuts. A majority of these cuts will be in rock. In accordance with ODOT's Geotechnical Bulletin Number 3 (GB-3) "Rock Cut Slope & "Catchment Design", Phase 1 rock cuts have been evaluated. For information pertaining to the rock cut slopes, please refer to our Report of Rock Cut Slopes dated November 29, 2006.

5.6 Construction Considerations

5.6.1 General

Based on the provided plans, profiles, and cross sections, the new roadway will consist of several cuts as well as embankment fills. All work should be performed in accordance with ODOT CMS, (Current Edition). Special care should be taken to ensure that the requirements of the CMS are met so that stable embankments are constructed.

5.6.2 Subgrade Preparation

Silt (A-4b) was encountered at the existing ground surface at several boring locations in the Phase 1 area. Whenever silt is encountered at the subgrade level, it should be overexcavated to at least three feet below subgrade and replaced with suitable, compacted fill. Additionally, no silt (A-4b) should be placed within three feet of subgrade in embankment fill sections.

It should be noted that borings drilled for the relocated Shumway Hollow Road Interchange encountered soils classified as elastic clay (A-7-5). Additionally, borings drilled for the Lucasville-Minford Road Interchange and the relocated Shumway Hollow Road Interchange encountered soils with liquid limits above 65. Although borings drilled for this portion of the roadway (station 416+00 to 509+50) did not encounter these soils, excavations may encounter these soils. Both soils classified as elastic clay (A-7-5) and soils having liquid limits in excess of 65 should not be used in embankment construction as per ODOT CMS Item 703.16. Caution should be used while excavating in these soils to determine the type of soil and its suitability as fill material.

5.6.3 New Embankment Construction

ODOT CMS Item 201 "Clearing and Grubbing" should be completed across the entire portion of the embankment foundation. The foundation should be compacted to at least 95% of the Standard Proctor value as outlined in ODOT CMS section 203.05. Also, it is recommended that the foundation soils be proof rolled (ODOT Item 204.06) prior to placement of any embankment materials. Any soft, yielding areas should be undercut to firm material and replaced with controlled, engineered fill. If seeps are encountered, spring drains should be installed to reduce the potential for the fill to become saturated in the future.

Locations of borrow areas are not known at this time. However, if glacial tills are utilized as the fill materials, any large durable cobbles or boulders greater than 8 inches in any dimension that cannot be broken down should be segregated and not be

incorporated into the lift. In addition, any soil classified as silt (A-4b) should not be used as fill.

5.6.4 Embankment Drainage

All embankments and side hill fills shall have a drainage layer in the lower portion of the fill, at the foundation soil-fill interface. This drainage layer shall consist of a minimum of six feet of free-draining, durable, rock fill as defined in ODOT CMS, Item 203.6.C and Item 703.16.C.

If springs or seeps are encountered during construction, the flow shall be collected within the embankment drainage layer or directed to the embankment drainage layer with a ditch or a trench drain. A typical trench drain shall be a minimum of one foot in width, with a depth and grade suitable for positive drainage. Six inches of concrete sand (ODOT Item 703.02) shall be placed in the bottom of the trench, then a six-inch diameter, fabric-wrapped, perforated PVC pipe shall be placed on top of the sand layer. The trench shall then be backfilled to the surface with concrete sand.

All ponds that lie within the footprint of a planned embankment shall be drained, and all "muck" and unsuitable material removed. Ponds may require benching as set forth in ODOT CMS Item 203.05 or placement of a spring or seep drain prior to embankment fill placement. Ponds known to have a spring and requiring a spring drain are indicated as spring fed ponds on the plans.

5.6.5 Rock Excavation

It is anticipated that rock excavation will be required for the roadway and interchange construction as indicated in Section 5.4.10. In addition, sandstone bedrock was encountered at the subgrade elevations indicated in Section 5.4.1. Due to the hardness of the rock and the length of the proposed alignment, blasting may be needed to excavate the rock. Rock encountered at these and any other locations at the subgrade elevation should be undercut and replaced by controlled, engineered fill as outlined by ODOT CMS Item 204.05.

5.6.6 GB-2 Special Benching

The roadway cross-sections were analyzed in accordance with Ohio Department of Transportation's Office of Geotechnical Engineering Geotechnical Bulletin 2 (GB-2), released February 7, 2006. ODOT specifications require that any side hill fill on an existing slope steeper than 8:1 should be benched according to the ODOT CMS Item 203.05. The ODOT Office of Geotechnical Engineering recommends special benching on existing slopes 4:1 or steeper. Special benching is used to improve the constructability and stability of the proposed embankment. Special benching is always shown on the cross-sections in the project plans and never on a typical cross-

section. Whenever special benching is used, Plan Note G110 from the ODOT Location and Design Manual, Volume 3, needs to be included in the general notes.

Construction along the proposed alignment will consist of side hill fills ranging from less than 2 feet to more than 60 feet. The existing slopes range from nearly flat to steeper than 3:1. In some cases, the existing ground surface is steeper than 4:1 and relatively thin, sliver fills will be placed. The bedrock and overburden interface is a common avenue for water during the wet winter months. In two places along the alignment, relatively thin fills will be placed in areas that will likely experience seepage and ground water infiltration from this interface. These fills are in areas that may be too thin or small to effectively place the free draining durable rock layer. In order to increase the performance of the embankments in these areas, special benching and drainage is recommended from station 415+00 to 416+00 and station 456+50 to 457+50.

All embankment fill should be placed in accordance with ODOT CMS Items 203.6: Spreading and Compacting and 203.7: Compaction and Moisture Requirements. The majority of the material to be excavated from the benching operation should be acceptable material for embankment fill. However, the material may have excessive moisture contents and may require moisture adjustments prior to compaction.

5.7 Excavation and Groundwater Considerations

Seepage and groundwater conditions are variable across the Phase 1 location. The Contractor should be prepared to keep excavations reasonably dry, such as with sumping and pumping. The Contractor should also be prepared to deal with unexpected seepage and precipitation that enters any excavations. Please refer to section 4.3 of this document, and the boring logs in Appendix A for more information concerning seepage and groundwater levels.

Excavations deeper than 4 feet must be laid back or braced to protect workers entering the excavations. All excavations should be constructed in accordance with applicable local, state, and federal safety regulations including the current OSHA Excavation and Trench Safety Standards (29 CFR Part 1926). Slopes or bracing for excavations 20 feet or more in depth must be designed by a registered professional engineer.

5.8 Geotechnical Design Checklists

The geotechnical design checklist applicable to this report is included in Appendix D.

6.0 CLOSING REMARKS

You are encouraged to discuss with us any questions you may have concerning the findings, conclusions, and recommendations presented in this report. Please do not hesitate to call if we can be of further assistance.

Sincerely,

DLZ OHIO, INC.



Steven J. Riedy
Geotechnical Engineer



Pete Nix, P.E.
Geotechnical Division Manager

M:\proj\0121\3070.03\Stability Analyses\Documents\Embankments\Mainline Roadway Embankments\SCI-823 Mainline Embankment Report sjr.doc



APPENDIX A

Boring Location Plan
General Information - Drilling Procedures and Logs of Borings
Legend - Boring Log Terminology
Boring Logs – Eighty-three (83) Borings

STATION 352+00 BEGIN
RELOCATED SHUHWAY
HOLLOW ROAD INTERCHANGE;
SEE RELOCATED SHUHWAY
HOLLOW INTERCHANGE REPORT

STATION 352+00
BEGIN PHASE 1



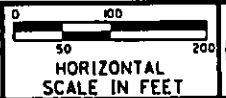
1/8

SCI-823-6.81

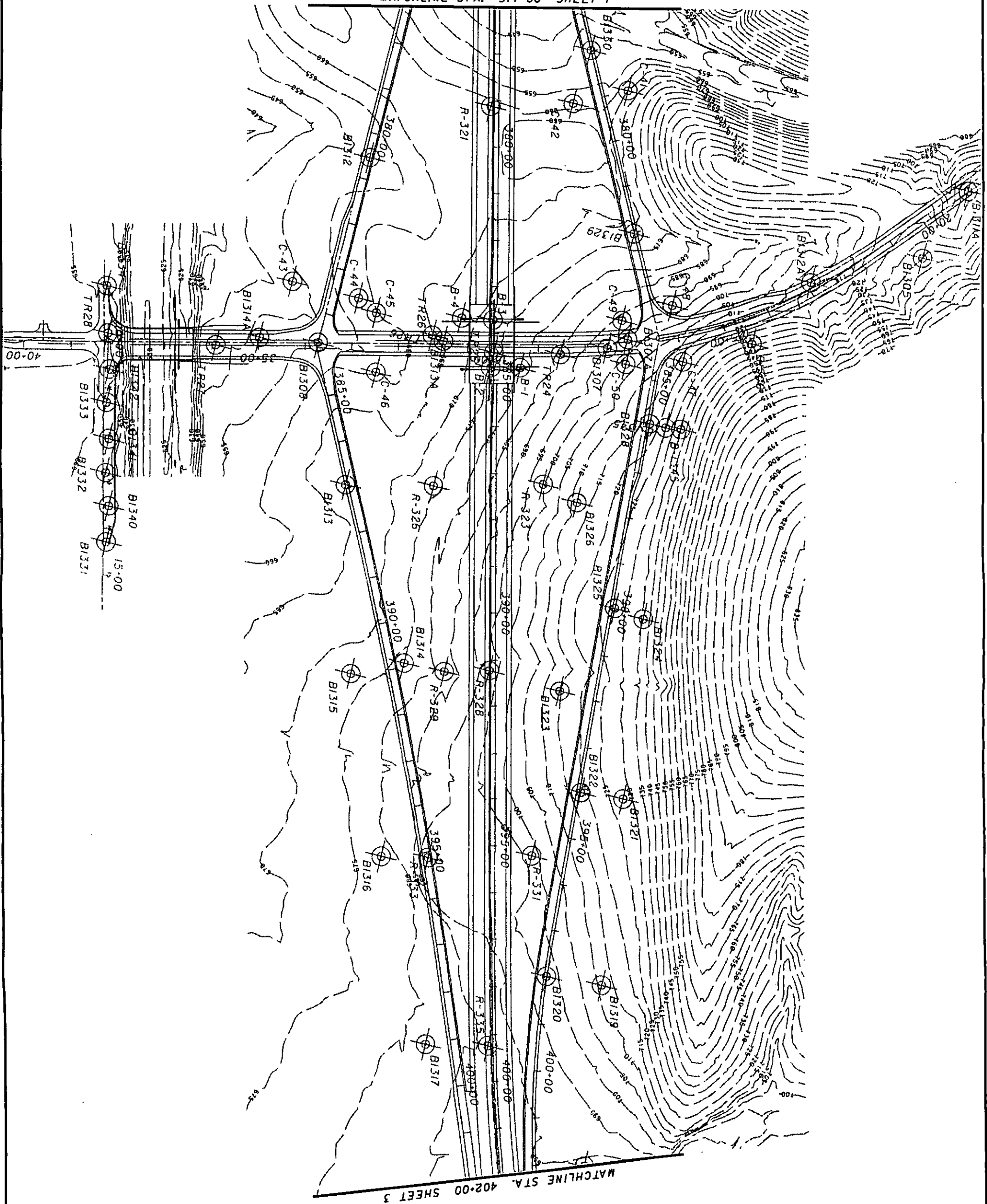
BORING PLAN

SR 823 STA. 352+00 TO STA. 377+00

DRAWN
RLS
CHECKED



MATCHLINE STA. 377+00 SHEET 1



MATCHLINE STA. 402+00 SHEET 3



2/8

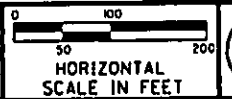
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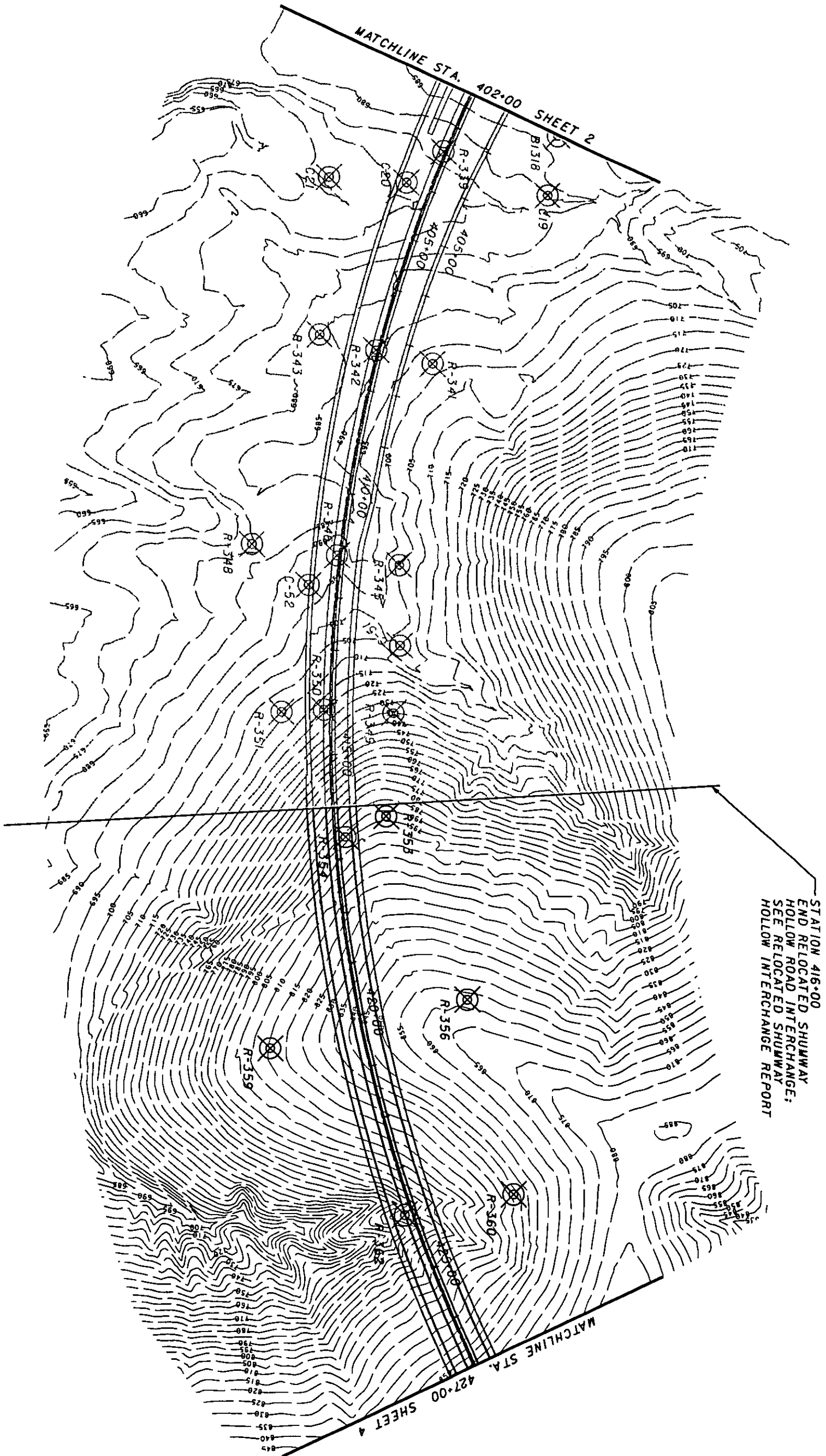
BORING PLAN

SR 823 STA. 377+00 TO STA. 402+00

DRAWN
RLS

CHECKED





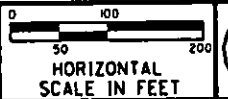
3/8

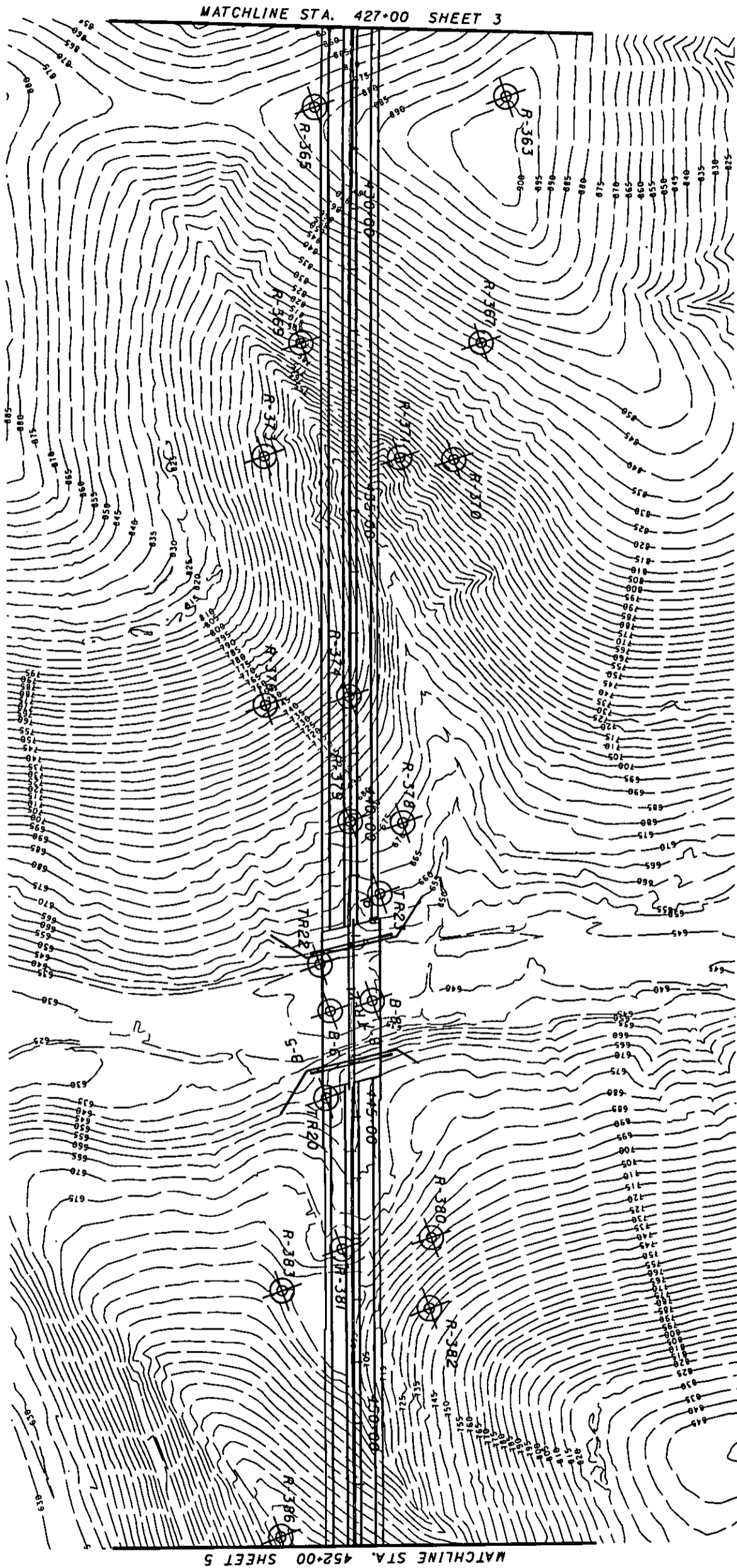
SCI-823-6.81

BORING PLAN
SR 823 STA. 402+00 TO STA. 427+00

DRWN
RLS

CHECKED

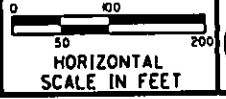


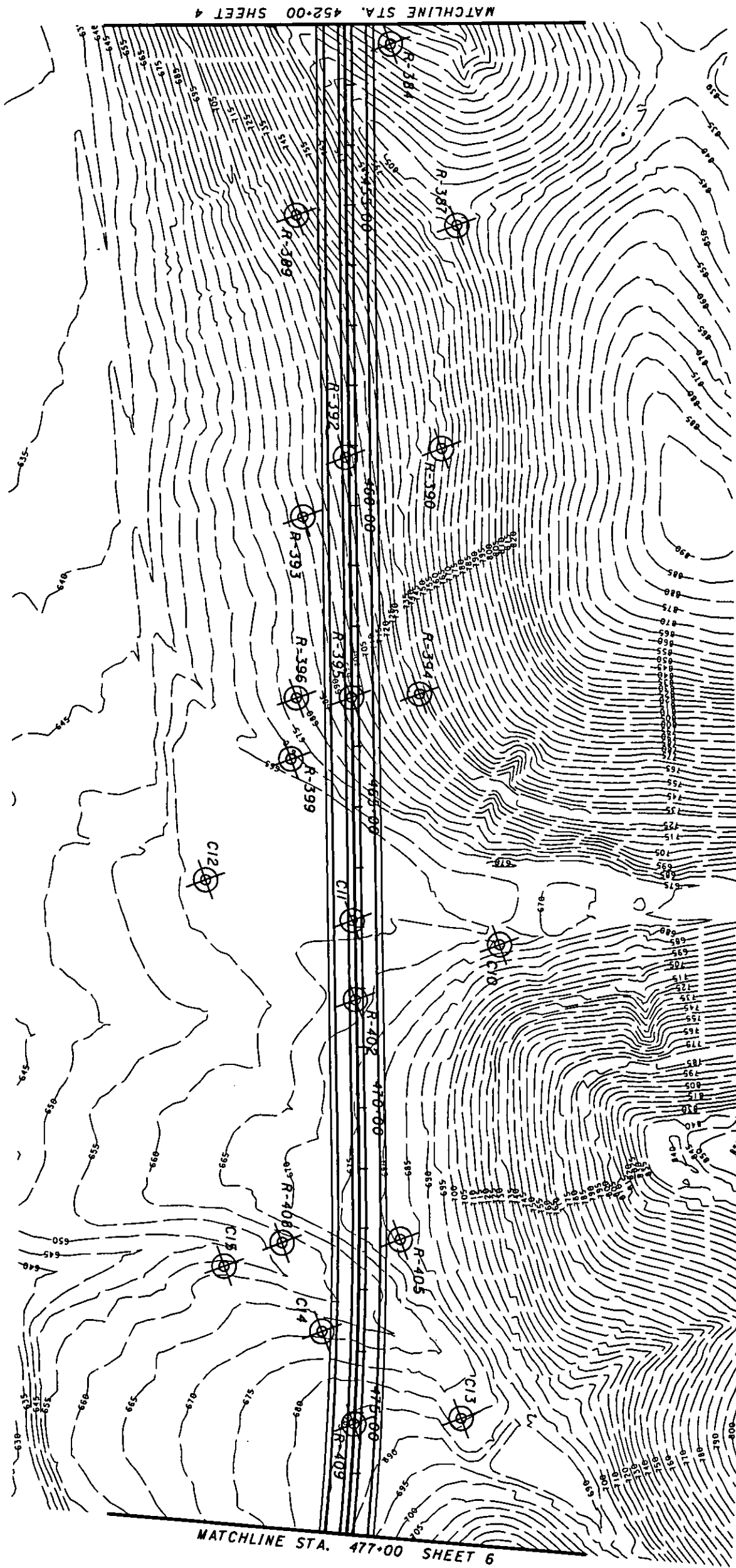


SCI-823-6.81

SOIL PROFILE
SR 823 STA. 427+00 TO STA. 452+00

DRAWN
RLS
CHECKED

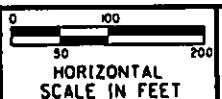


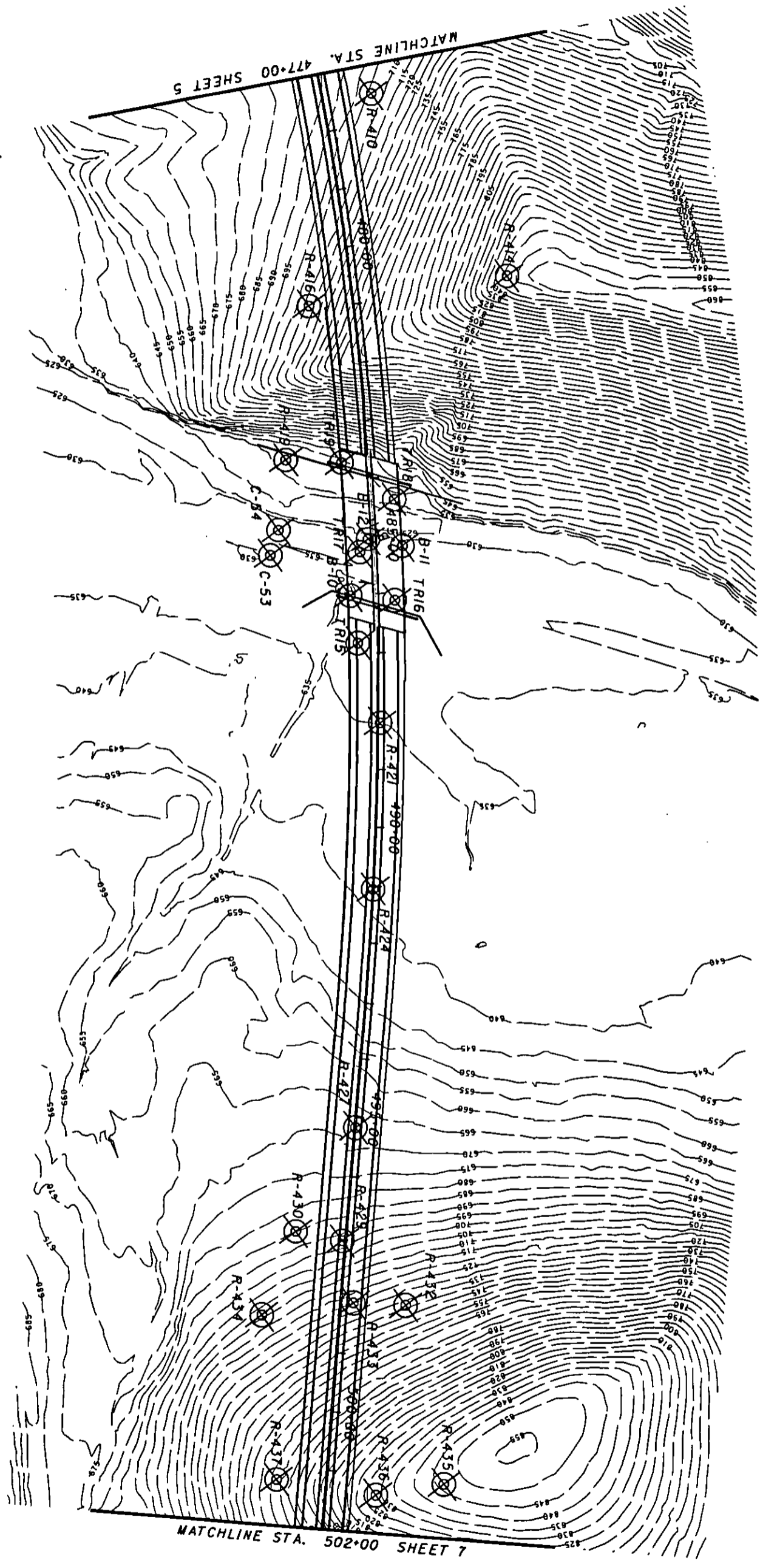


SCI-823-6.81

SOIL PROFILE
SR 823 STA. 452+00 TO STA. 477+00

DRAWN
RLS
CHECKED

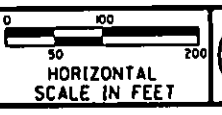




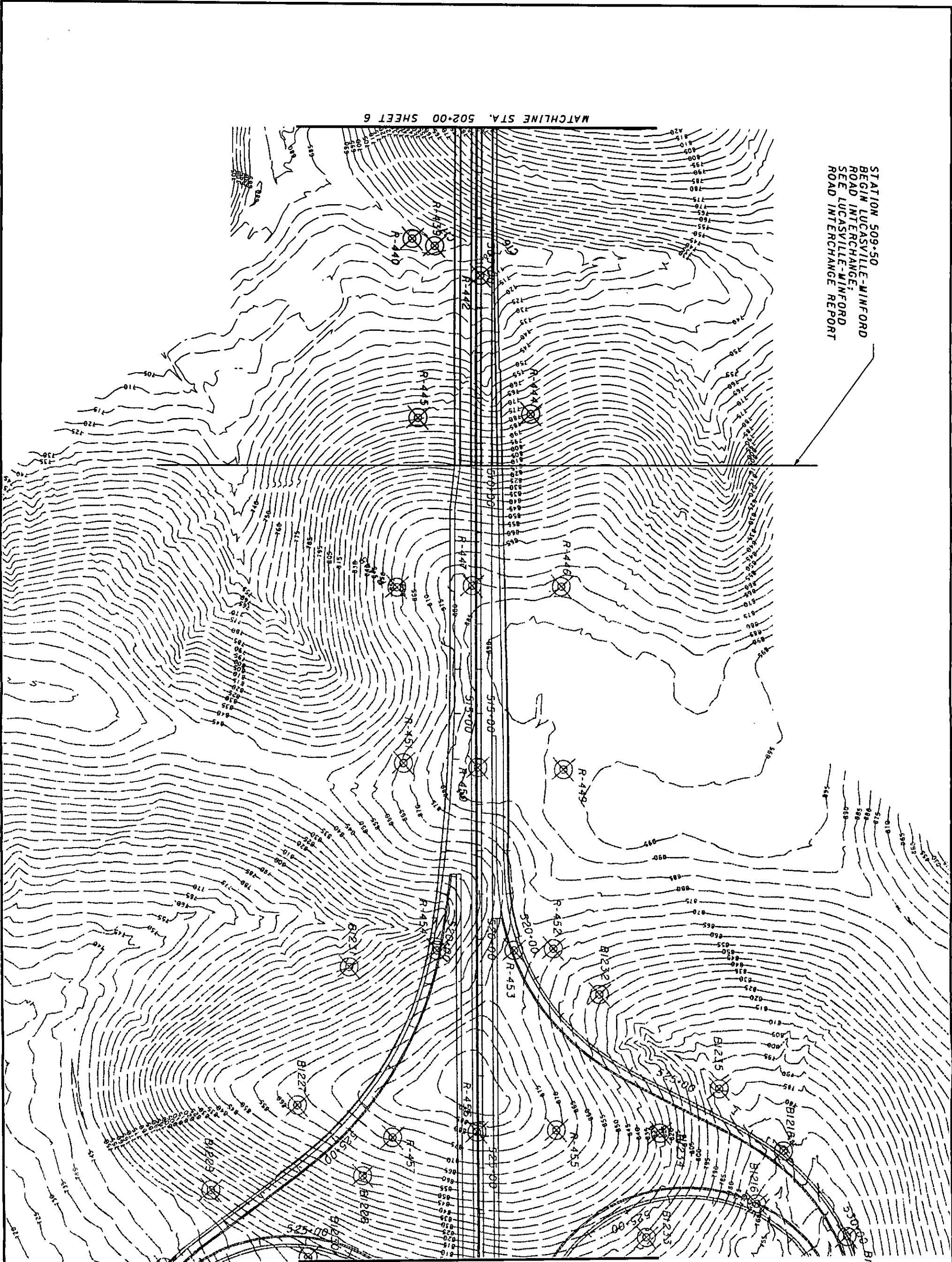
SCI-823-6.81

BORING PLAN
SR 823 STA. 477+00 TO STA. 502+00

DRWN
RLS
CHECKED



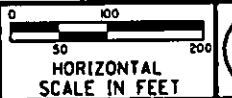
STATION 509+50
BEGIN LUCASVILLE-WINFORD
ROAD INTERCHANGE
SEE LUCASVILLE-WINFORD
ROAD INTERCHANGE REPORT

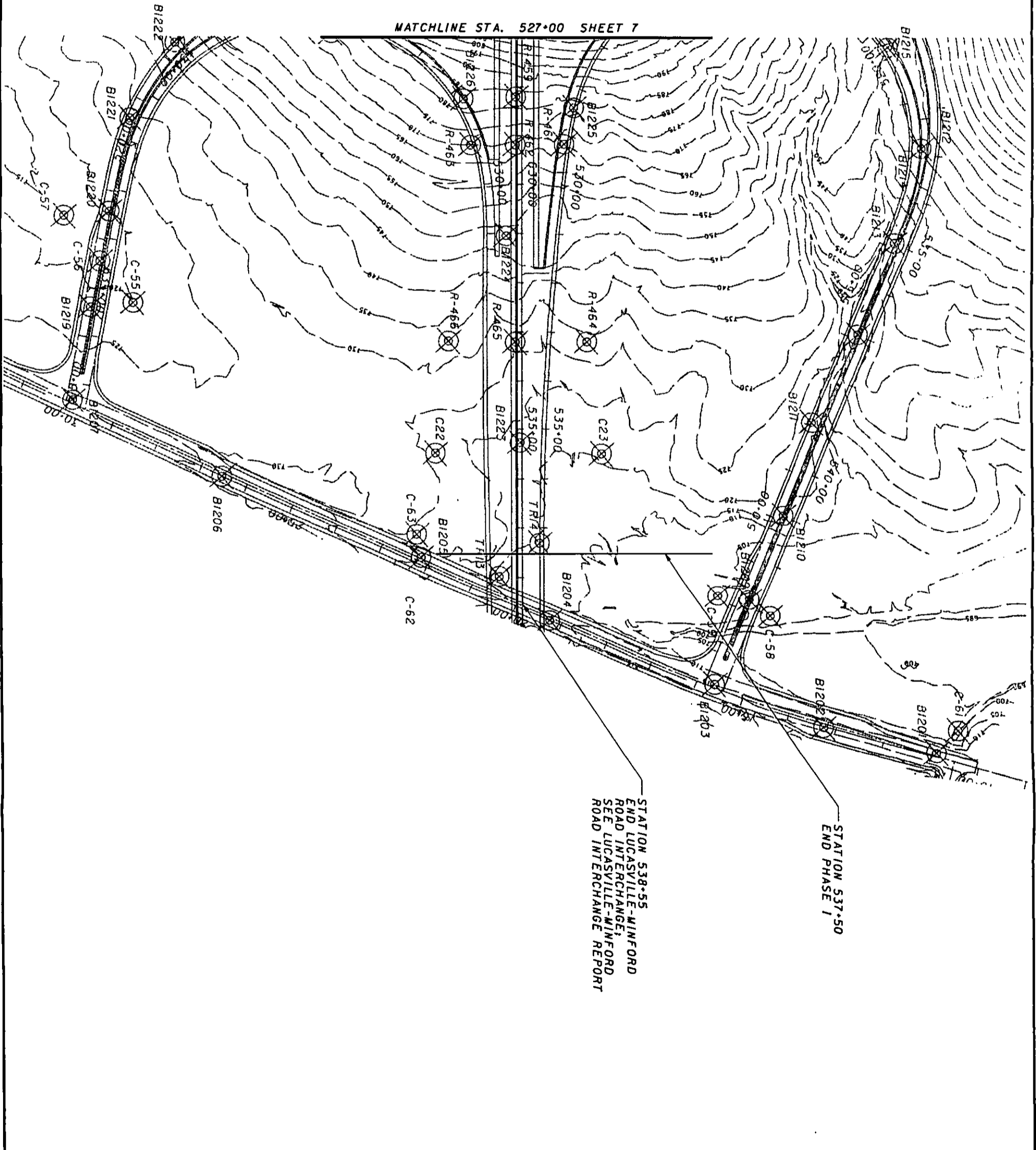


SCI-823-6.81

BORING PLAN
SR 823 STA. 502+00 TO STA. 527+00

DRAWN
RLS
CHECKED





GENERAL INFORMATION DRILLING PROCEDURES AND LOGS OF BORINGS

Drilling and sampling were conducted in accordance with procedures generally recognized and accepted as standardized methods of investigation of subsurface conditions concerning geotechnical engineering considerations. Borings were drilled with either a truck-mounted or ATV-mounted drill rig.

Drive split-barrel sampling was performed in 1.5 foot increments at intervals not exceeding 5 feet. In the event the sampler encountered resistance to penetration of 6 inches or less after 50 blows of the drop hammer, the sampling increment was discontinued. Standard penetration data were recorded and one or more representative samples were preserved from each sampling increment.

In borings where rock was cored, NXM or NQ size diamond coring tools were used.

In the laboratory all samples were visually classified by a geotechnical engineer. Moisture contents of representative fine-grained soil samples were determined. A limited number of samples, considered representative of foundation materials present, were selected for performance of grain-size analyses and plasticity characteristics tests. The results of these tests are shown on the boring logs.

The boring logs included in the Appendix have been prepared on the basis of the field record of drilling and sampling, and the results of the laboratory examination and testing of samples. Stratification lines on the boring logs indicating changes in soil stratigraphy represent depths of changes approximated by the driller, by sampling effort and recovery, and by laboratory test results. Actual depths to changes may differ somewhat from the estimated depths, or transitions may occur gradually and not be sharply defined. The boring logs presented in this report therefore contain both factual and interpretative information and are not an exact copy of the field log.

Although it is considered that the borings have disclosed information generally representative of site conditions, it should be expected that between borings conditions may occur which are not precisely represented by any one of the borings. Soil deposition processes and natural geologic forces are such that soil and rock types and conditions may change in short vertical intervals and horizontal distances.

Soil/rock samples will be stored at our laboratory for a period of six months. After this period of time, they will be discarded, unless notified to the contrary by the client.

LEGEND – BORING LOG TERMINOLOGY

Explanation of each column, progressing from left to right

1. Depth (in feet) – refers to distance below the ground surface.
2. Elevation (in feet) – is referenced to mean sea level, unless otherwise noted.
3. Standard Penetration (N) – the number of blows required to drive a 2-inch O.D., 1-3/8 inch I.D., split-barrel sampler, using a 140-pound hammer with a 30-inch free fall. The blows are recorded in 6-inch drive increments. Standard penetration resistance is determined from the total number of blows required for one foot of penetration by summing the second and third 6-inch increments of an 18-inch drive.

50/n – indicates number of blows (50) to drive a split-barrel sampler a certain number of inches (n) other than the normal 6-inch increment.
4. The length of the sampler drive is indicated graphically by horizontal lines across the "Standard Penetration" and "Recovery" columns.
5. Sample recovery from each drive is indicated numerically in the column headed "Recovery".
6. The drive sample location is designated by the heavy vertical bar in the "Sample No., Drive" column.
7. The length of hydraulically pressed "Undisturbed" samples is indicated graphically by horizontal lines across the "Press" column.
8. Sample numbers are designated consecutively, increasing in depth.

9. Soil Description

- a. The following terms are used to describe the relative compactness and consistency of soils:

Granular Soils – Compactness

<u>Term</u>	<u>Blows/Foot Standard Penetration</u>
Very Loose	0 – 4
Loose	4 – 10
Medium Dense	10 – 30
Dense	30 – 50
Very Dense	over 50

Cohesive Soils – Consistency

<u>Term</u>	<u>Unconfined Compression tons/sq.ft.</u>	<u>Blows/Foot Standard Penetration</u>	<u>Hand Manipulation</u>
Very Soft	less than 0.25	below 2	Easily penetrated by fist
Soft	0.25 – 0.50	2 – 4	Easily penetrated by thumb
Medium Stiff	0.50 – 1.0	4 – 8	Penetrated by thumb with moderate pressure
Stiff	1.0 – 2.0	8 – 15	Readily indented by thumb but not penetrated
Very Stiff	2.0 – 4.0	15 – 30	Readily indented by thumb nail
Hard	over 4.0	over 30	Indented with difficulty by thumb nail

- b. Color – If a soil is a uniform color throughout, the term is single, modified by such adjective as light and dark. If the predominant color is shaded by a secondary color, the secondary color precedes the primary color. If two major and distinct colors are swirled throughout the soil, the colors are modified by the term "mottled".

- c. Texture is based on the Ohio Department of Transportation Classification System. Soil particle size definitions are as follows:

<u>Description</u>	<u>Size</u>	<u>Description</u>	<u>Size</u>
Boulders	Larger than 8"	Sand – Coarse	2.0 mm to 0.42 mm
Cobbles	8" to 3"	– Fine	0.42 mm to 0.074 mm
Gravel – Coarse	3" to ¾"	Silt	0.074 mm to 0.005 mm
– Fine	¾" to 2.0 mm	Clay	smaller than 0.005 mm

- d. The main soil component is listed first. The minor components are listed in order of decreasing percentage of particle size.
- e. Modifiers to main soil descriptions are indicated as a percentage by weight of particle sizes.

trace	0 to 10%
little	10 to 20%
some	20 to 35%
"and"	35 to 50%

- f. Moisture content of **cohesionless soils** (sands and gravels) is described as follows:

<u>Term</u>	<u>Relative Moisture or Appearance</u>
Dry	No moisture present
Damp	Internal moisture, but none to little surface moisture
Moist	Free water on surface
Wet	Voids filled with free water

- g. The moisture content of **cohesive soils** (silts and clays) is expressed relative to plastic properties.

<u>Term</u>	<u>Relative Moisture or Appearance</u>
Dry	Powdery
Damp	Moisture content slightly below plastic limit
Moist	Moisture content above plastic limit but below liquid limit
Wet	Moisture content above liquid limit

10. Rock Hardness and Rock Quality Designation

- a. The following terms are used to describe the relative hardness of the **bedrock**.

<u>Term</u>	<u>Description</u>
Very Soft	Permits denting by moderate pressure of the fingers. Resembles hard soil but has rock structure. (Crushes under pressure of fingers and/or thumb)
Soft	Resists denting by fingers, but can be abraded and pierced to shallow depth by a pencil point. (Crushes under pressure of pressed hammer)
Medium Hard	Resists pencil point, but can be scratched with a knife blade. (Breaks easily under single hammer blow, but with crumbly edges.)
Hard	Can be deformed or broken by light to moderate hammer blows. (Breaks under one or two strong hammer blow, but with resistant sharp edges.)
Very Hard	Can be broken only by heavy and in some rocks repeated hammer blows.

- b. Rock Quality Designation, RQD – This value is expressed in percent and is an indirect measure of rock soundness. It is obtained by summing the total length of all core pieces which are at least four inches long, and then dividing this sum by the total length of the core run.

- 11. Gradation – when tests are performed, the percentage of each particle size is listed in the appropriate column (defined in Item 9c).
- 12. When a test is performed to determine the natural moisture content, liquid limit moisture content, or plastic limit moisture content, the moisture content is indicated graphically.
- 13. The standard penetration (N) value in blows per foot is indicated graphically.

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring B-7 Location: Sta. 444+00.8, 65.4 ft. LT of SR 823 CL *As Per Plan Date Drilled: 06/15/06

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○			
							DESCRIPTION												
0.2	658.0						Topsoil - 2"												
	657.8	8			1		Medium dense gray SANDY SILT (A-4a), damp. (Decomposed Sandstone)												
2.5	655.5	18 23	13				Medium hard to hard gray SANDSTONE; very fine to fine grained, moderately to highly weathered, argillaceous, micaceous, fossiliferous (trace fossils), massive bedding, highly fractured. @ 2.5'-9.3', rust staining. @ 2.5'-5.0', broken zone. @ 3.5', lost water circulation. @ 6.5', qu = 7,966 psi. @ 8.7'-8.9', high angle fracture.												
5				Core 120"	Rec 97"	RQD 76%													
10						R-1													
12.5	645.5						Bottom of Boring - 12.5'												
15																			
20																			
25																			
30																			

FILE: 0121-3070-03 [11/24/2006 2:03 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring B-10

Location: Sta. 486+01.5, 43.8' RT of SR 823 CL

Date Drilled: 06/28/06

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (prior to coring) 6.0' (inside hollowstem augers, includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	632.6																		
0.3	632.3						Topsoil - 3"												
		4				1.5	Stiff brown SILT (A-4b), little clay, trace to little fine sand; damp.	0	0	-	10	74	16						
		5	4	14															
		3				--													
5		3	2	17															
		2				2.0	@ 6.0'-7.5', soft, wet.	0	0	-	11	74	15						
		2	2	13															
8.5	624.1	10					Severely weathered gray SANDSTONE.												
9.5	623.1	50/2	6				Medium hard to hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, laminated to thinly bedded, moderately fractured.												
10																			
15		Core 120"	Rec 116"		RQD 87%	R-1	@ 16.5', qu = 10,393 psi.												
19.5	613.1						Bottom of Boring - 19.5'												
20																			
25																			
30																			

FILE: 0121-3070-03 [11/24/2006 2:03 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring C-11

Location: Sta. 466+89.0, 5.0 ft. RT of SR 823 CL

Date Drilled: 06/28/06

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 0.5' (includes drilling water, inside hollowstem augers)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0.1	663.9																			
	663.8						Topsoil - 1"													
		6				3.0	Very stiff brown SILT (A-4b), some clay, trace fine to coarse sand; damp.	0	2	-	4	68	26							
		5	4	17	1															
		3				3.0	@ 4.5', little gravel.													
5.0	658.9	2	10	13	2		Very stiff brown SANDY SILT (A-4a), little clay, little gravel; damp.													
		4	8	13	3	3.0		16	14	-	15	42	13							
		4	12	13	4	3.0														
10.5	653.4	4	10	18	5	4.5+	Hard gray SILT (A-4b), little clay, trace fine to coarse sand; damp.	0	0	-	7	77	16							
13.0	650.9						Soft to medium hard gray SANDSTONE; very fine to fine grained, slightly weathered, laminated to very thinly bedded, slightly fractured.													
15		Core 60"	Rec 53"		RQD 73%	R-1														
18.0	645.9						Bottom of Boring - 18.0'													
20																				
25																				
30																				

FILE: 0121-3070-03 [11/24/2006 2:04 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring C-53

Location: Sta. 485+25.3, 177.8 ft. RT of SR 823 CL

Date Drilled: 09/14/06

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 3.0' Water level at completion: None (prior to coring) 2.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40							
0.3	630.5						Topsoil - 4" Stiff brown SILT (A-4b), little to some clay, trace fine sand; damp to moist. Loose brownish gray SANDY SILT (A-4a), trace to little clay, little coarse sand, trace gravel; moist. @ 6.0', becomes brown. Soft brown SANDSTONE; very fine grained, highly weathered to decomposed. Soft to medium hard gray SANDSTONE; fine grained to very fine grained, slightly weathered, thinly bedded to medium bedded, moderately fractured. @ 8.0', 8.1', 8.5', low angle fractures.														
1.0	630.2	1		1		1.5															
2.0		2	10																		
3.0	627.5	1		2		-															
5.0		3	14																		
6.5	624.0	5		3		-															
7.0	623.5	50/3	7																		
10.0		Core 60"	Rec 60"		RQD 85%	R-1															
12.0	618.5						Bottom of Boring - 12.0'														
15.0																					
20.0																					
25.0																					
30.0																					

FILE: 0121-3070-03 (11/24/2006 2:06 PM)

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring C-54

Location: Sta. 484+80.3, 161.7 ft. RT of SR 823 CL

Date Drilled: 09/12/06

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.7' (includes drilling water)	GRADATION					STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40								
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay							
0	633.3						Topsoil - 5" Medium stiff to stiff brown SANDY SILT (A-4a), trace to little clay, trace gravel; damp to moist. @ 8.5'-10.0', little gravel.														
0.4	632.9	3		1																	
		3																			
		5																			
5		9		2																	
		4																			
		4																			
		5		3		1.0															
		5																			
		6																			
		2		4																	
10		2																			
		2																			
		2																			
11.0	622.3						Medium hard to hard gray SANDSTONE; fine grained, slightly weathered, thinly bedded to medium bedded, moderately fractured.														
15		Core 60"	Rec 60"	RQD 78%	R-1																
16.0	617.3						Bottom of Boring - 16.0'														
20																					
25																					
30																					

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-356

Location: Sta. 420+12.6, 210.3 ft. LT of SR 823 CL

Date Drilled: 7/23/04 to 7/28/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0.3	857.6																		
0.3	857.3						Topsoil - 4"/3" soil removed before drilling												
		3				4.5+	Hard brown and gray SILT AND CLAY (A-6a), trace fine to coarse sand; damp.												
		5	18	1															
		8				4.5+													
		9	18	2															
		10				4.5+													
		4																	
		10	18	3															
8.0	849.6						Soft gray SHALE; highly weathered.												
		24																	
		31	18	4															
		30																	
		35	18	5															
		48																	
		32																	
		50/4		6															
15							Soft brown and gray SHALE; highly weathered, arenaceous, micaceous, thinly laminated to thinly bedded, broken (contains numerous healed fractures); contains few arenaceous laminations.												
15.5	842.1	Core 60"	Rec 57"	RQD 60%	R-1	*50													
20																			
25		Core 120"	Rec 102"	RQD 48%	R-2	*116	@ 24.0', gray. @ 24.9'-27.0', qu = 1,886 psi; SDI = 21.9%. @ 27.0'-29.0', SILTSTONE lens.												
29.0	828.6						Medium hard gray SANDSTONE; highly weathered.												
30																			

FILE: 0121-3070-03 [11/24/2006 2:08 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-359

Location: Sta. 420+27.7, 176.5 ft. RT of SR 823 CL

Date Drilled: 7/27/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	815.9						No Topsoil Hard brown and gray CLAY (A-7-6), little silt, trace fine sand, trace gravel; damp.													
		8 13 14	18	1		4.5+														
		9 15 17	18	2		4.5+														
5.5	810.4						Severely weathered gray SILTSTONE.													
		31 50/3	9	3			Very soft grayish brown SILTSTONE; decomposed to highly weathered, slightly arenaceous, thinly laminated to thinly bedded, highly fractures with low angle clay filled fractures. @ 10.0'-10.2' ironstone band @ 11.0'-12.4', qu = 339 psi, SDI = 5.9%.													
7.0	808.9																			
10		Core 72"	Rec 72"	RQD 85%	R-1	*30	@ 14.4'-14.8', ferric zone, possible filled fractures. Hard brown and gray SANDSTONE; very fine to fine grained, highly weathered to moderately weathered, massively bedded, argillaceous, micaceous, slightly pyritic, slightly to moderately fractured; contains few argillaceous laminations. @ 17.9', low angle clay filled fracture. @ 19.5', low angle clay filled fracture.													
14.8	801.1																			
		Core 120"	Rec 120"	RQD 89%	R-2	*214	@ 23.6', low angle clay filled fracture. @ 26.7'-26.8', high angle fracture.													
20																				
		Core 120"	Rec 120"	RQD 99%	R-3	*380														
25																				
30																				

FILE: 0121-3070-03 [11/24/2006 2:08 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-359

Location: Sta. 420+27.7, 176.5 ft. RT of SR 823 CL

Date Drilled: 7/27/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
60	755.9						Hard brown and gray SANDSTONE; very fine to fine grained, highly weathered to moderately weathered, massively bedded, argillaceous, micaceous, slightly pyritic, slightly fractured to unfractured; contains few argillaceous laminations. @ 64.2'-64.7', qu = 11,633 psi.											
65																		
70		Core 120"	Rec 120"	RQD 100%	R-7	*509												
75							@ 92.2'-92.3', ferric band.											
80		Core 120"	Rec 120"	RQD 100%	R-8	*416												
85																		
90		Core 120"	Rec 120"	RQD 98%	R-9	*481												

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-360

Location: Sta. 424+20.5, 196.2 ft. LT of SR 823 CL

Date Drilled: 7/22/04 to 7/23/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 11.3' - 11.7' Water level at completion: None (Prior to coring) 11.0' (at the start of 7/23/04) 9.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press/Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	836.1						No Topsoil/1.4' soil removed before drilling Very stiff brown and gray SANDY SILT (A-4a), some gravel, little clay; contains sandstone fragments; damp.												
		10 11 12	18	1		2.25													
		9 13 15	18	2		2.5			27	13	-	16	26	18					
5	830.6							Very stiff brown and gray SILT (A-4b); some clay; little fine to coarse sand; damp.											
5.5		9 11 11	18	3		2.25				2	3	-	15	58	22				
8.0	828.1							Hard brown CLAY (A-7-6); damp.											
		11 12 16	9	4		4.5+													
10	825.6						Severely weathered dark brown CLAYSHALE.												
10.5		20 28 33	18	5															
		24 40 50/5	17	6															
15	820.6						Very soft to soft brown and gray CLAYSHALE; highly weathered to decomposed, micaceous, thinly laminated to thinly bedded, moderately fractured .												
15.5		Core 90"	Rec 90"	RQD 86%	R-1	*17													
20							Soft black SHALE; moderately to highly weathered, carbonaceous, arenaceous, thinly laminated to thinly bedded, fractured; contains few arenaceous laminations, very fissile. @ 25.0', 25.6', 25.9', low angle weathered fracture. @ 25.9'-26.4', high angle fracture.												
22.4	813.7																		
25		Core 120"	Rec 120"	RQD 75%	R-2	*53													
29.0	807.1						Medium hard gray SHALE; moderately to highly weathered.												
30																			

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-360 Location: Sta. 424+20.5, 196.2 ft. LT of SR 823 CL Date Drilled: 7/22/04 to 7/23/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 11.3' - 11.7' Water level at completion: None (Prior to coring) 11.0' (at the start of 7/23/04) 9.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
30	806.1						Medium hard gray SHALE; moderately to highly weathered, micaceous, thinly laminated to thinly bedded. @ 32.3', contains small arenaceous clasts. Hard gray very fine grained SANDSTONE; moderately weathered, micaceous, very thinly bedded to thinly bedded, (gradational change). @ 33.7'-34.7', contains ferric zones. Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, thinly bedded to thickly bedded, (gradational change), slightly fractured to unfractured. @ 35.0'-35.5', qu = 11,937 psi. @ 38.1', pyritic.									
32.8	803.3															
34.5	801.6															
35																
40		Core 120"	Rec 120"	RQD 98%	R-3	*194										
45																
50		Core 120"	Rec 120"	RQD 100%	R-4	*549										
55																
60		Core 120"	Rec 120"	RQD 100%	R-5	*506	@ 58.0'-58.2', very hard ferric band.									

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-363 Location: Sta. 428+11.8, 258.2 ft. LT of SR 823 CL Date Drilled: 7/20/04 to 7/22/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 62.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40	
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
0.2	899.6						Topsoil - 2" Hard brown SILT AND CLAY (A-6a), trace gravel; damp. Soft brown and gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous. Soft black SHALE; fine grained, moderately weathered, carbonaceous; contains coal stringers. Soft gray CLAYSHALE; moderately weathered, sulfur stains. Soft gray SHALE; fine grained, moderately weathered. Soft dark gray SHALE; highly weathered, carbonaceous, micaceous, arenaceous, laminated to thinly bedded, highly fractured. @ 28.4'-29.4', SDI = 34.6% @ 28.7'-30.0', qu = 1,652 psi, SDI = 20.1%.								
	899.4	6				1		4.5+							
		7	9	17											
3.0	896.6	5	11	20	18	2									
5		17	11	20	18	3									
10		25	48	40	18	4									
		8	27	35	18	5									
13.0	886.6	13	39	45	18	6A									
14.5	885.1	22	33	38	16	7									
15		37	38	40	13	8									
18.0	881.6	25	50/4	10		9									
20		37	38	50/4	18	10									
25.0	874.6														
30		Core 60"	Rec 56"	RQD 70%	R-1										

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-363

Location: Sta. 428+11.8, 258.2 ft. LT of SR 823 CL

Date Drilled: 7/20/04 to 7/22/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 62.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40			
30	869.6						DESCRIPTION Soft to medium hard dark gray SHALE; highly weathered, carbonaceous, micaceous, laminated to thinly bedded, slightly fractured; contains few arenaceous laminations and dust. @ 40.0', arenaceous. Medium hard gray and dark gray SANDSTONE; very fine to fine grained, moderately weathered, micaceous, argillaceous, laminated to thinly bedded; contains turbidity beds, moderately fractured to unfractured.										
35		Core 120"	Rec 120"	RQD 75%	R-2												
40																	
45		Core 120"	Rec 120"	RQD 97%	R-3												
47.2	852.4																
50																	
55		Core 120"	Rec 120"	RQD 98%	R-4												
60																	

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-363

Location: Sta. 428+11.8, 258.2 ft. LT of SR 823 CL

Date Drilled: 7/20/04 to 7/22/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 62.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - PL ————— LL Blows per foot - ○ 10 20 30 40							
60	839.6						<p>DESCRIPTION</p> <p>Hard gray and dark gray SANDSTONE; very fine to fine grained, moderately weathered, micaceous, laminated to thinly bedded, turbidity bedded with carbonaceous stringers. @ 60.2'-63.3', fine to medium grained.</p> <p>Medium hard dark gray SILTSHALE; moderately weathered, arenaceous, carbonaceous, laminated to thinly bedded, highly to moderately fractured; contains arenaceous laminations and dust. @ 63.3'-64.7', 65.0'-66.8', highly fractured to broken with high angle fractures.</p> <p>@ 75.3'-76.9', fine to coarse grained ferric layers. @ 76.1'-76.4', carbonaceous shale layers</p> <p>Hard light gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, thinly bedded to thickly bedded. @ 76.9'-77.5', rust stained.</p>														
63.3	836.3																				
65		Core 120"	Rec 116"	RQD 63%	R-5																
70																					
75		Core 120"	Rec 119"	RQD 98%	R-6																
76.9	822.7																				
80																					
85		Core 120"	Rec 118"	RQD 98%	R-7																
90																					

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-367

Location: Sta. 432+20.4, 217.6 ft. LT of SR 823 CL

Date Drilled: 7/28/04 to 7/29/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 46.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay								
0.2	845.8																				
	845.6						Topsoil - 2"														
		3				4.5+	Hard brown and gray SILT (A-4b), little to some clay; little fine to coarse sand, trace gravel; damp.														
		4																			
		5	18		1																
		13																			
5		14				4.25															
		15	18		2																
		12																			
		14				4.5+															
		15	18		3			5	4	-	11	61	20								
		4																			
10		8				4.5+															
		12																			
10.5	835.3						Severely weathered gray and brown CLAYSHALE.														
		12																			
		50/5	9		5																
		20																			
		50/4	10		6																
15.0	830.8						Medium hard brown CLAYSHALE; highly weathered to decomposed, laminated, broken to highly fractured with low angle rust stained fractures. @ 15.4'-15.6', high angle fractures.														
		Core 60"	Rec 60"	RQD 50%	R-1	*9															
20							@ 21.5'-21.8', rust stained beds. @ 22.5'-24.0', qu = 8,200 psi, SDI = 71.0%.														
25		Core 120"	Rec 120"	RQD 88%	R-2	*409	@ 29.1'-30.5', sandstone seam.														
30																					

FILE: 0121-3070-03 [11/24/2006 2:09 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-369

Location: Sta. 432+21.3, 80.1 ft. RT of SR 823 CL

Date Drilled: 8/2/04

to 8/3/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 8.4' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
30	775.9						MEDIUM HARD TO HARD GRAY SANDSTONE; very fine to fine grained, slightly to moderately weathered, micaceous, laminated to thinly bedded, slightly fractured to unfractured. @ 41.0' low angle fractures									
35																
40		Core 120"	Rec 120"	RQD 97%	R-4	*530										
45																
50		Core 120"	Rec 120"	RQD 100%	R-5	*513										
55																
60		Core 120"	Rec 120"	RQD 100%	R-6	*430										

FILE: 0121-3070-03 [11/24/2006 2:10 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-370

Location: Sta. 434+14.1, 171.9 ft. LT of SR 823 CL

Date Drilled: 7/29/04 to 7/30/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 5.6' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - PL ————— LL Blows per foot - ○ 10 20 30 40						
60	727.7						<p>DESCRIPTION</p> <p>Hard gray SANDSTONE; very fine to fine grained, slightly weathered, slightly argillaceous, micaceous, slightly fractured to unfractured. @ 61.2'-62.4', medium hard to hard, medium grained, moderately weathered, poorly cemented.</p> <p>@ 66.8', occasional calcareous layer. @ 67.2', occasional pyrite nodule.</p> <p>@ 75.1', few arenaceous laminations.</p> <p>@ 83.0', pyritic.</p>													
65				Core 120"	Rec 120"	RQD 100%		R-7	*420											
75				Core 120"	Rec 120"	RQD 100%		R-8	*570											
85				Core 84"	Rec 84"	RQD 100%		R-9	*569											
90.0	697.7								Bottom of Boring - 90.0'											

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-370

Location: Sta. 434+14.1, 171.9 ft. LT of SR 823 CL

Date Drilled: 7/29/04 to 7/30/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 5.6' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○		
							DESCRIPTION											
90	697.7																	
95																		
100																		
105																		
110																		
115																		
120																		

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-371

Location: Sta. 434+11.0, 82.9 ft. LT of SR 823 CL

Date Drilled: 7/29/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.9' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40	
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
0	770.5						Topsoil - 5"/2.2' soil removed before drilling Medium dense to dense brown SANDY SILT (A-4a), trace to little clay, trace gravel; contains sandstone fragments; damp.								
0.4	770.1	9				1									
		12 15	18												
		15				2									
		15 15	16												
5							Medium hard brown SANDSTONE; very fine to fine grained, highly weathered, broken to highly fractured; contains clay filled fractures. @ 10.6', 11.4', thin clay seam. @ 13.4', 13.8', 14.2', low angle clay coated fracture.								
5.5	765.0														
		Core 90"	Rec 90"	RQD 59%	R-1	*424									
10							Medium hard to hard gray SANDSTONE, very fine to fine grained, moderately weathered, argillaceous, micaceous, massive, slightly fractured to unfractured. @ 15.7'-15.9', high angle fractures, clay seam. @ 19.9', low angle clay filled fracture. @ 20.0', 20.7', low angle rust stained fractures. @ 22.0', slightly weathered.								
14.6	755.9														
		Core 120"	Rec 120"	RQD 94%	R-2	*455									
20							@ 26.2', occasional pyrite nodule.								
		Core 120"	Rec 120"	RQD 93%	R-3	*328									
25															
30															

FILE: 0121-3070-03 [11/24/2006 2:10 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-371

Location: Sta. 434+11.0, 82.9 ft. LT of SR 823 CL

Date Drilled: 7/29/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.9' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
60	710.5						DESCRIPTION Hard gray SANDSTONE; very fine to fine grained, moderately to slightly weathered, argillaceous, pyritic, massively bedded, slightly fractured. @ 63.0'-65.9', very pyritic.									
65		Core 120"	Rec 120"	RQD 100%	R-7	*411										
70																
75		Core 120"	Rec 120"	RQD 100%	R-8	*562	@ 86.7', low angle fractures. @ 87.5'-85.6', rust stained.									
80																
85		Core 84"	Rec 78"	RQD 93%	R-9	*498										
90.0	680.5						Bottom of Boring - 90.0'									

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-373

Location: Sta. 434+08.2, 141.1 ft. RT of SR 823 CL

Date Drilled: 8/4/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetro- meter (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
30	737.5						<p>Soft to hard gray SANDSTONE; very fine to fine grained, interbedded with SHALE, highly to moderately weathered, arenaceous, argillaceous, laminated to medium bedded, moderately fractured; contains argillaceous SHALE zones are very friable. @ 31.2'-31.6', qu = 10,759 psi.</p> <p>Soft to medium hard brown and gray SHALE; highly weathered to decomposed, thinly laminated to thinly bedded, arenaceous, highly fractured.</p> <p>Medium hard gray fine to medium grained SANDSTONE; moderately weathered, argillaceous, medium bedded, poorly cemented, highly fractured.</p> <p>Medium hard to hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, thinly bedded to thickly bedded, turbidity bedded, highly fractured; contains few argillaceous laminations.</p> <p>Hard gray SANDSTONE; very fine to fine grained, moderately to slightly weathered, argillaceous, massively bedded; contains few argillaceous laminations.</p> <p>@ 56.1', pyritic.</p>												
35.3	732.2																		
38.3	729.2																		
39.5 40	728.0	Core 120"	Rec 120"	RQD 28%	R-4														
45																			
50 50.6	716.9	Core 120"	Rec 118"	RQD 58%	R-5														
55																			
60		Core 120"	Rec 120"	RQD 100%	R-6														

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-373

Location: Sta. 434+08.2, 141.1 ft. RT of SR 823 CL

Date Drilled: 8/4/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % -		Blows per foot -			
60	707.5																		
65																			
70		Core 120"	Rec 120"	RQD 100%	R-7														
75.0	692.5																		
80																			
85																			
90																			

DESCRIPTION

Hard gray SANDSTONE; very fine to fine grained, moderately to slightly weathered, argillaceous, pyritic massively bedded; contains few argillaceous laminations.

@ 63.6'-64.4', high angle fractures.

Bottom of Boring - 75.0'

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-374

Location: Sta. 438+05.1, 1.5 ft. RT of SR 823 CL

Date Drilled: 8/4/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 17.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
0	723.0						<p>DESCRIPTION</p> <p>Topsoil - 5"/12" soil removed before drilling.</p> <p>Loose to medium dense light brown SANDY SILT (A-4a); contains sandstone fragments; damp.</p> <p>Severely weathered brown SANDSTONE argillaceous.</p>									
0.4	722.6	2 4	4	16	1											
3.0	720.0	15 26 31		17	2											
5		17 44 50/4		16	3											
8.5	714.5						<p>Soft to medium hard brown SANDSTONE; very fine to fine grained, highly weathered to decomposed, thinly bedded, micaceous, argillaceous, broken.</p>									
10		Core 78"	Rec 56"	RQD 22%	R-1											
15																
17.6	705.4						<p>Medium hard to hard gray SANDSTONE; moderately to highly weathered, laminated to medium bedded, contains few to moderate argillaceous laminations.</p> <p>@ 25.2'-27.4', broken and highly weathered; contains abundant argillaceous laminations.</p> <p>Hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, thinly bedded to medium bedded, argillaceous, micaceous.</p>									
20		Core 120"	Rec 116"	RQD 41%	R-2											
25																
27.4	695.6															
30		Core 120"	Rec 119"	RQD 41%	R-3											

FILE: 0121-3070-03 [11/24/2006 2:10 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-376

Location: Sta. 438+19.8, 139.9 ft. RT of SR 823 CL

Date Drilled: 8/4/04 to 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40	
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
0.3	749.2						Topsoil - 3"/1.5' soil removed before drilling Dense brown SANDY SILT (A-4a); contains sandstone fragments; dry to damp. Severely weathered brown SANDSTONE argillaceous.								
		12 15 15	18			1									
3.0	746.5	37 50/4	10			2									
5		20 50/4	10			3									
8.0	741.5	Core 36"	Rec 34"	RQD 25%	R-1		Very soft to soft brown and gray SANDSTONE interbedded with SHALE; very fine to fine grained, highly weathered to decomposed, argillaceous, moderately to highly fractured. @ 12.1'-13.4', SDI = 92.8%. @ 15.0', gray and brown. @ 16.6'-18.5', qu = 385 psi, SDI = 23.3. @ 19.8' to 20.7', brown, highly weathered.								
10															
15		Core 120"	Rec 113"	RQD 38%	R-2										
20															
25		Core 120"	Rec 117"	RQD 43%	R-3	*310	@ 26.7' to 27.2', high angle rust stained fracture. @ 27.3', gray with small argillaceous clasts.								
30															

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-376

Location: Sta. 438+19.8, 139.9 ft. RT of SR 823 CL

Date Drilled: 8/4/04

to 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
60	689.5						DESCRIPTION												
60.5	689.0							Soft to medium hard brown SANDSTONE; very fine to fine grained, highly weathered to decomposed, argillaceous, highly fractured.											
65		Core 120"	Rec 117"	RQD 98%	R-7	*461													
70																			
71.0	678.5						Bottom of Boring - 71.0'												
75																			
80																			
85																			
90																			

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-380

Location: Sta. 446+99.5, 130.0 ft. LT of SR 823 CL

Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 6.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	700.0																		
0-1		8				1	4.5+												
1-2		10 12	18																
2-3		14				2	4.0												
3-4		20 21	18																
4-5		13				3	4.25												
5-6		12 8	18																
6-7		9				4	4.5+												
7-8		15 13	18					18	4	-	11	46	21						
8-9		5				5	4.5+												
9-10		6 11	18					19	4	-	13	45	19						
10-11		14				6	4.5+												
11-12		50/3	9																
12-13																			
13-14																			
14-15																			
15-16	684.0																		
16-17																			
17-18																			
18-19																			
19-20																			
20-21		Core 84"	Rec 84"		RQD 96%	R-1													
21-22																			
22-23																			
23-24																			
24-25																			
25-26																			
26-27																			
27-28																			
28-29																			
29-30		Core 120"	Rec 120"		RQD 93%	R-2													

FILE: 0121-3070-03 [11/24/2005 2:11 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-382

Location: Sta. 448+15.2, 126.4 ft. LT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 9.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	733.4						No topsoil Severely weathered brown SANDSTONE, argillaceous.												
		12 11 11	18	1															
		36 50/4	10	2															
5.5	727.9						Medium hard to hard brown SANDSTONE; very fine to fine grained, moderately to highly weathered, laminated to medium bedded, micaceous, argillaceous, broken. @ 7.4', moderately fractured. @ 8.4', 12.3', low angle fractures. @ 8.9'-9.0', 10.5', 10.6', 10.8', high angle fractures. @ 12.3'-13.0', broken zone. @ 13.1', 13.3', 13.7', 14.2', 16.3' 16.4', 17.2', low angle fractures.												
10		Core 90"	Rec 90"	RQD 50%	R-1														
17.6	715.8	Core 120"	Rec 120"	RQD 91%	R-2			Hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, micaceous, pyritic, laminated to medium bedded, slightly fractured. @ 17.6', 22.5', 22.7', low angle fractures. @ 22.9'-26.9', gray and brown. @ 23.8', thin ironstone nodule. @ 23.7', low angle highly weathered fracture. @ 25.2', 25.4', 26.5', 26.8', low angle highly weathered fractures.											
25		Core 120"	Rec 120"	RQD 95%	R-3														
30																			

FILE: 0121-3070-03 [11/28/2006 2:45 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-382

Location: Sta. 448+15.2, 126.4 ft. LT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 9.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40			
30	703.4						Hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, micaceous, slightly fractured, pyritic, laminated to medium bedded. @ 32.0', slightly weathered. @ 36.4', 36.8', 39.0', 39.4', 39.7', low angle fractures in argillaceous laminations. @ 43.0', slightly fractured to unfractured.										
35																	
		Core 120"	Rec 120"	RQD 95%	R-4												
40																	
45																	
		Core 120"	Rec 120"	RQD 100%	R-5												
50																	
		Core 24"	Rec 24"	RQD 100%	R-6												
55.0	678.4						Bottom of Boring - 55.0'										
60																	

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-384

Location: Sta. 452+33.5, 74.6 ft. LT of SR 823 CL

Date Drilled: 8/9/04 to 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: Not reported	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
0	760.5																			
0.4	760.1						Topsoil - 5"													
		36 20 27	18			1	Severely weathered brown SANDSTONE, argillaceous.													
		23 25 29	18			2														
5		26 50/2	8			3														
7.0	753.5						Medium hard to hard brown SANDSTONE; very fine to fine grained, decomposed to highly weathered, broken, argillaceous, contains thin clay seams.													
10		Core 72"	Rec 60"	RQD 36%	R-1		@ 14.2', fractured. @ 10.4'-12.5', qu = 2,102 psi, SDI = 73.9%. @ 14.5'-14.7', 16.0'-16.5', broken decomposed zones.													
15							@ 16.8'-17.4', high angle fractures. @ 17.2'-17.4', decomposed zone.													
20		Core 120"	Rec 120"	RQD 63%	R-2		@ 17.9', low angle fracture. @ 18.0'-22.0', contains moderate to abundant argillaceous laminations, decomposed to highly weathered. @ 22.1'-22.7', medium to coarse grained, poorly cemented.													
22.7	737.8						Hard gray SANDSTONE; very fine to fine grained, highly weathered, argillaceous, contains thin clay seams.													
25							@ 24.1', 24.3', 26.3', 27.8', low angle fractures. @ 25.3', slightly weathered to moderately weathered. @ 25.5'-27.0', qu = 9,350 psi, SDI = 93.2%.													
30		Core 120"	Rec 114"	RQD 90%	R-3															

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03
 LOG OF: Boring R-386 Location: Sta. 451+83.9, 122.1 ft. RT of SR 823 CL Date Drilled: 8/10/04 to 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 42.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0	774.5						No topsoil/Drilled on old road cut Medium dense brown SANDY SILT (A-4a); contains sandstone fragments; dry.											
2		4	7	14	1													
3.0	771.5						Soft brown SANDSTONE; very fine to fine grained, decomposed, argillaceous, micaceous, (has soil like appearance), contains moderate to abundant argillaceous laminations.											
5		Core 72"	Rec 32"	RQD 0%	R-1													
9.0	765.5						Soft to medium hard brown SANDSTONE; very fine to fine grained, highly weathered, argillaceous, micaceous, contains moderate to abundant argillaceous laminations.											
10		Core 24"	Rec 22"	RQD 0%	R-2													
15		Core 36"	Rec 27"	RQD 14%	R-3		@ 14.4', brown and gray, highly weathered to moderately weathered.											
20		Core 120"	Rec 116"	RQD 47%	R-4													
25							@ 29.6', gray.											
29.6	744.9	Core 120"	Rec 120"	RQD 58%	R-5													

FILE: 0121-3070-03 [11/24/2006 2:12 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-386

Location: Sta. 451+83.9, 122.1 ft. RT of SR 823 CL

Date Drilled: 8/10/04 to 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetro-meter (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 42.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - PL ————— LL Blows per foot - ○ 10 20 30 40			
90	684.5						MEDIUM HARD GRAY SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, pyritic, micaeous, contains abundant to moderate argillaceous laminations. @ 92.5'-93.9', qu = 8,768 psi, SDI = 91.8%.										
95		Core 72"	Rec 72"	RQD 75%	R12												
100.0	674.5						Bottom of Boring - 100.0'										
105																	
110																	
115																	
120																	

FILE: 0121-3070-03 [11/24/2006 3:13 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-387

Location: Sta. 455+35.9, 182.3 ft. LT of SR 823 CL

Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 23.7' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
30	770.7						<p>DESCRIPTION</p> <p>Medium hard to hard gray SANDSTONE; very fine to fine grained, highly weathered, argillaceous, micaceous, very thinly bedded to medium bedded, broken to highly fractured.</p> <p>@ 38.0', 38.3', interbedded with shale.</p> <p>@ 41.5'-43.2', 47.5'-49.1', contains moderate argillaceous laminations.</p> <p>@ 44.3'-45.3', qu = 8,852 psi, SDI = 97.3%.</p> <p>@ 51.5'-51.6', 52.9'-53.5', 54.4'-58.0', abundant argillaceous laminations.</p>									
		Core 120"	Rec 120"	RQD 51%	R-4											
45		Core 120"	Rec 120"	RQD 61%	R-5											
55		Core 120"	Rec 120"	RQD 48%	R-6											
58.0	742.7															
58.8	741.9															
60																

FILE: 0121-3070-03 [11/24/2005 2:12 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-387

Location: Sta. 455+35.9, 182.3 ft. LT of SR 823 CL

Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 23.7' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
60	740.7						<p>DESCRIPTION</p> <p>Hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, very thin bedded to medium bedded, moderate to few fractures, contains calcareous layers. @ 62.2'-63.4', qu = 9,395 psi, SDI = 96.9%.</p> <p>@ 67.2'-67.7', high angle fracture with pyrite.</p> <p>@ 73.5', pyritic.</p> <p>@ 84.0', 86.9', 87.5', 88.9', low angle fractures.</p>													
65		Core 120"	Rec 116"	RQD 69%	R-7															
70																				
75		Core 120"	Rec 118"	RQD 67%	R-8															
80																				
85		Core 120"	Rec 120"	RQD 66%	R-9															
90																				

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-387 Location: Sta. 455+35.9, 182.3 ft. LT of SR 823 CL Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 23.7' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ----- LL Blows per foot - ○ 10 20 30 40							
90	710.7						Hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, very thinly bedded to medium bedded, moderately fractured, contains thin clay seams, and calcareous layers. @ 92.4', 96.5', 97.9', 98.1', 99.0', low angle fractures. @ 110.0', moderately to slightly weathered. @ 114.7'-115.1', qu = 9,618 psi. @ 116.0'-117.5', SDI = 70.6%.														
		Core 120"	Rec 116"	RQD 86%	R10																
		Core 120"	Rec 120"	RQD 82%	R11																
		Core 120"	Rec 120"	RQD 86%	R12																

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-387

Location: Sta. 455+35.9, 182.3 ft. LT of SR 823 CL

Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 23.7' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
120	680.7						DESCRIPTION Hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, very thinly bedded to medium bedded, moderately fractured, contains few argillaceous laminations and calcareous layers.													
125		Core 120"	Rec 119"	RQD 93%	R13															
130							Bottom of Boring - 131.0'													
131.0	669.7																			
135																				
140																				
145																				
150																				

FILE: 0121-3070-03 | 11/24/2006 2:13 PM |

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-389

Location: Sta. 455+17.0, 83.3 ft. RT of SR 823 CL

Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % -			
							DESCRIPTION							PL ——— LL			
0	727.4																
0.3	727.1						Topsoil - 3"/2.1' soil removed before drilling										
		9 17 34	18			1	Severely weathered brown SANDSTONE, argillaceous.										51
		24 33 36	18			2											
5																	
		50/2	2			3											69
7.0	720.4						Medium hard gray and brown SANDSTONE; very fine to fine grained, highly weathered, micaceous, argillaceous, thinly bedded to massive, highly fractured, typical fracture is low angle rust stained. @ 8.0'-9.5', SDI = 93.3%. @ 10.5'-10.9', qu = 6,641 psi.										
10		Core 72"	Rec 66"	RQD 33%	R-1	*227											
15																	
18.3	709.1	Core 120"	Rec 120"	RQD 82%	R-2	*484	Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, argillaceous, thinly bedded to massive, slightly fractured to unfractured. @ 20.0', low angle clay filled fracture. @ 21.1', low angle clay filled fracture.										
20																	
25																	
		Core 120"	Rec 120"	RQD 100%	R-3	*487											
30																	

FILE: 0121-3070-03 [11/24/2005 2:12 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-389 Location: Sta. 455+17.0, 83.3 ft. RT of SR 823 CL Date Drilled: 8/11/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 8.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40			
30	697.4						@ 29.7'-31.2', qu = 11,215 psi, SDI = 98.8%. Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, argillaceous, thinly bedded to massive, slightly fractured to unfractured.										
35																	
40		Core 120"	Rec 120"	RQD 100%	R-4	*597											
45							@ 43.0'-50.0', contains friable bands of very fine sandstone.										
45		Core 84"	Rec 84"	RQD 98%	R-5	*450	@ 44.6', low angle clay filled fracture. @ 44.7'-44.8', high angle clay filled fracture.										
50.0	677.4						Bottom of Boring - 50.0'										
55																	
60																	

FILE: 0121-3070-03 [11/24/2005 2:12 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-390

Location: Sta. 459+06.6, 153.1 ft. LT of SR 823 CL

Date Drilled: 8/11/04 to 8/12/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 19.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, %		Blows per foot					
							DESCRIPTION						PL ————— LL Blows per foot - ○ 10 20 30 40								
0	763.3						Topsoil - 12"														
1.0	762.3	6					Medium dense to dense brown SANDY SILT (A-4a), little gravel, trace clay; contains sandstone fragments; damp.														
		9	18	1																	
		7					@ 6.0', very dense.														
5		17	18	2																	
		21					Severely weathered brown SANDSTONE argillaceous, micaceous.														
		30	18	3																	
8.0	755.3						Medium hard brown SANDSTONE; very fine to fine grained, highly weathered, micaceous, thinly bedded to thickly bedded, moderately fractured.														
		9	18	4																	
10		25	18				Hard gray SANDSTONE; very fine to fine grained, highly to moderately weathered, micaceous, thinly bedded to thickly bedded; contains few to moderate argillaceous laminations.														
		48	18	5																	
		7					@ 16.5', 16.7', 18.8', low angle clay filled fractures. @ 19.5', 21.7', low angle clay filled fractures. @ 19.2'-19.3', 21.3'-21.5', high angle fractures. @ 21.7'-21.8', fine to medium, poorly cemented sandstone. @ 24.8', massively bedded, moderately weathered.														
13.0	750.3	30	18	6																	
		50/2	1				Core 30" Rec 30" RQD 97% R-1 *208														
15																					
16.3	747.0						Core 120" Rec 120" RQD 46% R-2 *481														
20																					
25																					
30																					

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-390 Location: Sta. 459+06.6, 153.1 ft. LT of SR 823 CL Date Drilled: 8/11/04 to 8/12/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 19.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
30	733.3	Core 120"	Rec 120"	RQD 100%	R-3	*285	Hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, micaceous, argillaceous, massively bedded, slightly fractured to unfractured. @ 33.8'-47.0', occasional pyrite nodules.													
35																				
40		Core 120"	Rec 120"	RQD 100%	R-4	*661														
45																				
50		Core 120"	Rec 120"	RQD 100%	R-5	*481	@ 54.3'-54.8', very fine sandstone friable.													
55																				
60																				

FILE: 0121-3070-03 [11/24/2006 2:12 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-390

Location: Sta. 459+06.6, 153.1 ft. LT of SR 823 CL

Date Drilled: 8/11/04 to 8/12/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 19.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
60	703.3	Core 120"	Rec 120"	RQD 100%	R-6	*578	Hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, micaceous, argillaceous, massively bedded, slightly fractured to unfractured.													
65																				
70		Core 120"	Rec 120"	RQD 100%	R-7	*549														
75							@ 75.5'-76.3', 76.8'-85.5', very fine SANDSTONE, friable.													
80		Core 120"	Rec 120"	RQD 100%	R-8	550														
85																				
85.5	677.8						Bottom of Boring - 85.5'													
90																				

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-392

Location: Sta. 459+19.3, 6.3 ft. RT of SR 823 CL

Date Drilled: 8/3/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	711.9																			
0.5	711.4						Topsoil - 6"													
		8				1	Very stiff brown SANDY SILT (A-4a); contains sandstone fragments; dry to damp.													
		7	12																	
		9				2														
		16	18																	
5		25																		
		15				3	Severely weathered brown SANDSTONE, argillaceous.													
		17	18																	
		24																		
8.0	703.9					4														
		8																		
		18	18																	
10		29																		
		5				5	Hard to medium hard brown SANDSTONE; highly weathered, moderately to highly fractured, argillaceous, contains moderate to few argillaceous laminations. @ 12.2'-12.3' high angle fractures @ 12.4', 13.0', 13.1', 13.3', 13.8', 14.5', 14.8', 15.0' low angle fractures @ 12.5' gray @ 16.4'-16.5', interbedded with shale. @ 17.3', 17.6', 18.2', 18.3', 19.9', low angle rust stained fractures.													
		50/4	4																	
11.5	700.4																			
15																				
		Core 120"	Rec 120"																	
20																				
25																				
		Core 102"	Rec 102"																	
30.0	681.9						Bottom of Boring - 30.0'													

FILE: 0121-3070-03 [11/21/2005 2:12 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-392

Location: Sta. 459+19.3, 6.3 ft. RT of SR 823 CL

Date Drilled: 8/3/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40	
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
30	681.9						DESCRIPTION								
35															
40															
45															
50															
55															
60															

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-393

Location: Sta. 460+18.5, 78.2 ft. RT of SR 823 CL

Date Drilled: 8/3/04

to 8/4/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 4.0' Water level at completion: 14.5'	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	691.0																		
1.0	690.0	7 6 8	18	1			Topsoil - 12"												
5.0		11 13 11	18	2			Medium dense brown and gray SANDY SILT (A-4a), trace gravel; contains sandstone fragments; damp.												
		4 9 6	18	3															
8.0	683.0	7 12 10	18	4		4.0		Very stiff to hard brown and gray SANDY SILT (A-4a), trace to little gravel; contains sandstone fragments; damp.											
10.0		4 7 13	18	5		3.5													
15.0		10 20 26	18	6		4.5+			14	8	--	19	44	15					
18.0	673.0	7 17 16	18	7		4.5+	Medium dense brown SANDY SILT (A-4a); little gravel; damp. @ 18.5'-20.0', contains sandstone fragments.												
20.0		5 11 12	18	8															
25.0		4 7 9	18	9					15	8	--	14	42	21					
25.0		5 7 11	18	10															
30.0		11 13 16	18	11															
		6 16 10	18	12															

FILE: 0121-3070-03 [11/23/2005 2:13 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03
 LOG OF: Boring R-393 Location: Sta. 460+18.5, 78.2 ft. RT of SR 823 CL Date Drilled: 8/3/04 to 8/4/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 4.0' Water level at completion: 14.5'	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○				
30.0	661.0 661.0						Very stiff gray SILT (A-4b), some clay, trace fine to coarse sand, trace gravel; damp.													
35		2 2 3	18	13		3.0			1	1	-	8	60	30						
38.5	652.5	6					Severely weathered gray SANDSTONE, argillaceous, micaceous.													
40.0	651.0	16 44	18	14																
Bottom of Boring - 40.0'																				

FILE: 0121-3070-03 [11/24/2006 2:13 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-394

Location: Sta. 463+14.3, 111.8 ft. LT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 11.0' Water level at completion: None (prior to coring) 17.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
0	730.1						Topsoil - 12"												
1.0	729.1	15 17 29	18	1			Medium dense to dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; dry to damp.												
		10 19 34	18	2															
5		13 22 22	18	3															
		6 16 17	18	4															
10		7 11 14	18	5					9	5	--	17	53	16					
15		5 6 16	18	6															
15.5	714.6	15 15 50/3	12	7			Severely weathered brown and gray SANDSTONE, argillaceous, micaceous.												
		50/3	0	8															
19.5	710.6	Core 6"	Rec 6"	RQD 83%	R-1		Medium hard to hard brown and gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, very thinly bedded to medium bedded, moderately fractured with typical low angle rust and clay filled fractures. @ 20.6'-21.4', 29.0'-29.4', high angle highly weathered clay coated fractures. @ 23.6', thin clay seam. @ 24.5'-25.1', qu = 7,852 psi.												
20		Core 120"	Rec 120"	RQD 71%	R-2														
25																			
30							@ 29.4', gray, slightly weathered.												

FILE: 0121-3070-01 [11/24/2006 2:13 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-394

Location: Sta. 463+14.3, 111.8 ft. LT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 5'	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 11.0' Water level at completion: None (prior to coring) 17.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
30	700.1						Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, micaceous, very thinly bedded to medium bedded. @ 30.3', low angle rust stained fracture. @ 30.3'-30.5', limestone layer. @ 33.8'-34.3', qu = 7,616 psi.											
35		Core 120"	Rec 120"	RQD 100%	R-3													
40																		
45		Core 117"	Rec 117"	RQD 100%	R-4		@ 46.5', contains occasional pyritic nodule.											
49.7	680.4							Bottom of Boring - 49.7'										
55																		
60																		

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-395 Location: Sta. 463+19.1, 1.1 ft. RT of SR 823 CL Date Drilled: 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 13.5' Water level at completion: None	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	694.8																		
1.0	693.8	4					Topsoil - 12"												
		4					Loose brown SANDY SILT (A-4a), little clay, trace gravel; contains sandstone fragments; damp. @ 3.5', very dense.												
		5	18		1														
		14					Medium dense to dense brown SILT (A-4b), some fine to coarse sand, little clay; contains sandstone fragments; damp.												
		32			2														
5		23	18																
5.5	689.3																		
		7					Bottom of Boring - 20.0'												
		12			3														
		14	18																
		6			4														
		11																	
		13	18																
		5			5														
		8																	
		10			6														
		20	18																
		6			7														
		11																	
		21	18																
		10			8														
		22																	
20.0	674.8	17	18																
25																			
30																			

FILE: 0121-3070-03 [11/24/2006 2:13 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-396

Location: Sta. 463+18.6, 92.1 ft. RT of SR 823 CL

Date Drilled: 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 11.0' Water level at completion: None	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	678.9																			
1.0	677.9	3					Topsoil - 12"													
		3	18		1		Loose to medium dense brown and gray SANDY SILT (A-4a), "and" gravel, trace clay; contains sandstone fragments; damp.													
		5																		
		11																		
		16	18		2				47	7	--	7	32	7						
5		38																		
		16																		
		16	18		3															
		12																		
		4																		
		6	18		4															
10		8																		
		4																		
		7	18		5															
		10																		
13.0	665.9						Very stiff brown SILTY CLAY (A-6b), trace fine to coarse sand, trace gravel; damp.													
		3				3.5														
		4	18		6			4	3	--	5	59	29							
15		7																		
		5				4.0														
		9	18		7															
		9																		
18.0	660.9						Medium dense brown SANDY SILT (A-4a), trace clay, trace gravel; contains sandstone fragments; damp to moist.													
		5																		
		12	18		8															
20		16																		
20.5	658.4						Medium dense brown SILT(A-4b), some fine sand, little clay ; moist to wet.													
		4																		
		5	18		9			0	0	--	20	68	12	Non-Plastic						
		6																		
		8																		
		5	18		10		@ 23.5'-25.0', contains gray sand seams.													
25.0	653.9																			
		10																		
							Bottom of Boring - 25.0'													
30																				

FILE: 0121-3070-03 [11/24/2006 2:13 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-399

Location: Sta. 464+20.0, 102.8 ft. RT of SR 823 CL

Date Drilled: 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 8.5' Water level at completion: None (prior to coring) 6.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0.2	668.1						Topsoil - 2" Medium dense brown SILT (A-4b), some clay, little fine to coarse sand; moist. Stiff brown SILTY CLAY (A-6b), trace fine sand; contains sandstone fragments; damp. Medium dense brown SANDY SILT (A-4a), trace to little clay, trace gravel; contains sandstone fragments; damp. Hard to medium hard gray SANDSTONE, very fine to fine grained, highly to moderately weathered, thinly to thickly bedded, broken to slightly fractured. @ 17.5'-17.8', rust stained. @ 18.5'-18.7' rust stained, broken zone. Bottom of Boring - 20.0'											
	667.9	10 8	18	1														
		9																
		3 6	18	2					0	0	--	13	65	22	Non-Plastic	●		
5.5	662.6	5 8	6	3		1.5												
		11																
		2 2	18	4		2.0			0	0	--	1	59	40		●		
10		2																
10.5	657.6	2 3	18	5														
		6																
		5 5	18	6														
15		5																
		2 5	12	7														
17.5	650.6	50/4																
		Core 30"	Rec 30"		RQD 87%	R-1												
20.0	648.1																	
25																		
30																		

FILE: 0121-3070-03 | 11/24/2006 2:13 PM

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-402

Location: Sta. 468+18.5, 1.4 ft. RT of SR 823 CL

Date Drilled: 8/5/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 5.2' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
DESCRIPTION																				
0	668.1						Topsoil - 12"													
1.0	667.1	3	18	1		1.0	Stiff to very stiff brown and gray SILT (A-4b), little to some clay, trace to little fine to coarse sand, trace gravel; damp.	0	3	--	3	63	31							
		3																		
5		4	18	2		3.5			0	2	--	5	69	24						
		5																		
		2	18	3		1.5														
10		3	18	4		3.5														
		3																		
13.0	655.1						Severely weathered brown SANDSTONE.													
14.5	653.6	12	8	6			Medium hard to hard gray SANDSTONE, very fine to fine grained, highly to moderately weathered, thinly to thickly bedded, broken to slightly fractured.													
15		50/4							3	5	--	19	57	16						
20.0	648.1						Bottom of Boring - 20.0'													
25																				
30																				

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-405

Location: Sta. 472+18.3, 68.0 ft. LT of SR 823 CL

Date Drilled: 8/6/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 6.0' Water level at completion: None (prior to coring) None (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○								
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	10	20	30	40					
0	681.6																					
1.0	680.6	4			1		Topsoil - 12"															
		7 13	18				Medium dense to dense brown SANDY SILT (A-4a), trace gravel; contains sandstone fragments; damp.															
		11 21	18		2																	
5		8 12	18		3																	
		12 11	18																			
8.0	673.6	4			4	2.0		Very stiff brownish gray SILT AND CLAY (A-6a), trace fine sand, trace gravel; damp to moist.														
		4 5	18																			
10		3 3	18		5	2.5																
		3 5	18																			
15		3 5	18		6	2.5																
		2 5	18																			
		3 8	18		7	3.0																
20.0	661.6	3 8	16 18		8	4.0																
							Bottom of Boring - 20.0'															
25																						
30																						

FILES: 0121-3070-03 [11/21/2006 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-408

Location: Sta. 472+24.1, 128.2 ft. RT of SR 823 CL

Date Drilled: 8/6/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 2.5' Water level at completion: Not reported	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL ○ Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	656.6																		
-0.6	656.0	3					Topsoil - 6"												
		4	18	1			Loose to medium dense brown SILT (A-4b), little clay, trace gravel, trace fine to coarse sand; damp.	10	3	-	6	63	18						
		8					@ 3.5', contains sandstone fragments.												
5.0	651.6	14 50/5	12	2			Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, micaceous, very thinly bedded to medium bedded. @ 5.0'-5.2', brown, moderately weathered. @ 5.5'-5.7', broken, highly weathered, contains clay infilling.												
		Core 60"	Rec 60"	RQD 92%	R-1														
10																			
15		Core 120"	Rec 120"	RQD 98%	R-2		@ 13.7'-14.0', contains calcareous matrix.												
20.0	636.6						@ 18.9'-19.3', limestone layer.												
							Bottom of Boring - 20.0'												
25																			
30																			

FILE: 0121-3070-03 [11/24/2005 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-409

Location: Sta. 475+18.9, 1.2 ft. RT of SR 823 CL

Date Drilled: 8/9/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 5.0' Water level at completion: None	DESCRIPTION	GRADATION					STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40							
				Drive	Press / Core				% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay						
0	685.7																				
1.0	684.7	2						Topsoil-12"													
3.0		3	18			1		Loose to medium dense brown SANDY SILT (A-4a), trace to little gravel; damp.													
6.0		6	18																		
8.0		8				2		@ 11.0'-12.5', dense.													
9.0		9	18																		
13.0		13	18			3				13	16	--	13	43	15						
12.0		8				4															
11.0		12	18																		
10.0		17	18			5															
10.0		3																			
11.0		8	18																		
13.0	672.7							Loose brown and gray SILT (A-4b); moist.													
14.0		3				6			0	0	--	2	73	25							
15.0		4	18																		
16.0		2				7															
17.0		2	18																		
18.0	667.7							Medium dense brown SANDY SILT (A-4a), little gravel; contains sandstone fragments; damp to moist.													
19.0		6				8															
20.0	665.7	5	18																		
21.0		10																			
20.0								Bottom of Boring - 20.0'													

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-410

Location: Sta. 477+48.7, 83.4 ft. LT of SR 823 CL

Date Drilled: 8/9/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 10.0' Water level at completion: 20.0' (with augers removed)	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○			
							DESCRIPTION						PL ——— LL						
0	706.8						Topsoil - 12"												
1.0	705.8	2					Dense to very dense brown SILT (A-4b), trace to little gravel, trace clay; damp. @ 1.0'-2.5', loose.												
		2	18		1														
		8																	
		15	18		2				18	10	-	9	50	13					
5		20					Very stiff brown SILT AND CLAY (A-6a), trace gravel, little fine to coarse sand; damp.												
		39	18		3														
		9																	
		17	18		4														
10							Very stiff brown SILT AND CLAY (A-6a), trace gravel, little fine to coarse sand; damp.												
10.5	696.3	3																	
		3	18		5	2.0													
		5																	
		8	18		6	2.0													
15							Dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.												
		5																	
		11	18		7	3.25													
		16																	
		8	18		8	3.5													
20							Dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.												
		13																	
		18	18		9	3.5													
		26																	
23.0	683.8						Dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.												
		11																	
		14	18		10														
		17																	
25							Dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.												
		13																	
		17	18		11														
		17																	
		13					Dense brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.												
		16	18		12														
30																			

Z:\121-3070-03 [11/24/2006 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-410

Location: Sta. 477+48.7, 83.4 ft. LT of SR 823 CL

Date Drilled: 8/9/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 10.0' Water level at completion: 20.0' (with augers removed)	GRADATION						STANDARD PENETRATION (N)		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	LL	
30	676.8						Very stiff brown SILT (A-4b), some fine to coarse sand, little clay, trace gravel; contains sandstone fragments; damp.	1	14	-	10	57	18			
		7 9 12	18	13	3.75											
		10 13 17	18	14	3.25											
35.0	671.8						Bottom of Boring - 35.0'									
40																
45																
50																
55																
60																

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-414

Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL

Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
DESCRIPTION																		
0	846.9						No topsoil Severely weathered brown SANDSTONE.											
		16 50/4	10			1												
3.0	843.9						Medium hard to hard brown SANDSTONE; fine grained, moderately weathered, micaceous, slightly argillaceous, thinly bedded to medium bedded, contains small argillaceous clasts, broken to moderately fractured.											
		Core 120"	Rec 120"			RQD 83%	@ 4.9', 5.9', clay filled fractures. @ 8.0'-9.7', qu = 6,016 psi, SDI = 91.6%.											
						R-1	@ 6.8'-7.1', interbedded shale.											
		Core 120"	Rec 120"			RQD 80%	@ 15.0', brown and gray. @ 7.4'-25.0', highly fractured. @ 16.3', high angle clay filled fractures. @ 19.9'-20.1', interbedded shale.											
						R-2												
		Core 120"	Rec 120"			RQD 90%	@ 24.6', thin clay seam.											
						R-3												

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-414 Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
30.0	816.9 816.9						<p>Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, moderately to slightly fractured. @ 31.0', 32.7', high angle rust stained fractures. @ 31.4', low angle fracture.</p> <p>@ 36.1', 36.3', low angle highly weathered fractures. @ 36.3'-37.3', qu = 7,797 psi. @ 37.5'-38.0', qu = 7,698 psi.</p> <p>@ 47.1'-47.3', 47.5'-47.8', high angle highly weathered fractures with calcite on face.</p> <p>@ 59.0', 59.2', interbedded shale.</p>													
35																				
40		Core 120"	Rec 120"	RQD 98%	R-4															
45																				
50		Core 120"	Rec 120"	RQD 100%	R-5															
55																				
60		Core 120"	Rec 120"	RQD 100%	R-6															

FILE: 0121-3070-03 [11/24/2006 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-414

Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL

Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
60	786.9						DESCRIPTION									
65								Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, slightly fractured.								
70		Core 120"	Rec 120"	RQD 100%	R-7			@ 66.9', slightly pyritic.								
75							Hard brown fine grained SANDSTONE, moderately weathered, slightly argillaceous, thinly to thickly bedded, highly fractured to moderately fractured. @ 78.8'-79.6', broken zone. @ 79.9'- 79.4', lost recovery, fracture suspected.									
75.5	771.4	Core 120"	Rec 116"	RQD 72%	R-8											
80							Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, slightly fractured. @ 89.2'-95.5', moderate to few argillaceous laminations.									
82.8	764.1	Core 120"	Rec 120"	RQD 100%	R-9	*203		@ 87.5'-88.8', SDI = 78.3%.								
85																
90																

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-414 Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetro- meter (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
90	756.9						DESCRIPTION Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, slightly fractured. @ 109.5'-110.9', contains few argillaceous laminations, large concentration of pyrite. @ 119.0', pyritic inclusion.													
		Core 120"	Rec 120"	RQD 100%	R10															
		Core 120"	Rec 120"	RQD 100%	R11															
		Core 120"	Rec 120"	RQD 98%	R12															

FILE: 0121-3070-03 [11/24/2006 2:14 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-414 Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
120	726.9						DESCRIPTION Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, slightly fractured. @ 120.5'-121.6', qu = 11,804 psi. @ 126.0'-138.5', pyritic inclusions. @ 129.7'-130.0', broken with interbedded shale.											
125																		
130		Core 120"	Rec 120"	RQD 97%	R13													
135		Core 120"	Rec 120"	RQD 100%	R14													
140		Core 120"	Rec 120"	RQD 100%	R15													
145																		
150																		

FILE: 0121-3070-03 [11/24/2005 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-414

Location: Sta. 480+84.1, 267.1 ft. LT of SR 823 CL

Date Drilled: 8/12/04 to 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
150	696.9						DESCRIPTION Hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, slightly argillaceous, thinly bedded to thickly bedded, slightly fractured. @ 158.4'-158.5', high angle fracture. @ 170.5'-170.7', shale and clay filled fractures.									
155																
160		Core 120"	Rec 120"	RQD 100%	R16											
165																
170		Core 120"	Rec 120"	RQD 99%	R17											
173.0	673.9						Bottom of Boring - 173.0'									
175																
180																

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-416

Location: Sta. 480+94.2, 75.9 ft. RT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (1sf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 10.0' Water level at completion: None (prior to coring) 19.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	712.5																			
1.0	711.5	4					Topsoil - 12"													
		6 10	18				Dense to very dense brown SANDY SILT (A-4a), trace gravel, trace clay; contains sandstone fragments; damp. @ 1.0'-2.5', medium dense.													
		11 21	18																	
5		16 23	18																	
		6 13	18																	
		6 14	18																	
		15 35	18																	
15.0	697.5	53	18				Severely weathered brown SANDSTONE argillaceous, micaceous.													
17.0	695.5	8 50/3	6				Medium hard to hard brown SANDSTONE; very fine to fine grained, broken to moderately fractured, moderately weathered, micaceous, argillaceous. @ 21.0', high angle fracture. @ 20.5'-21.0', qu = 4,850 psi. @ 21.4', gray and brown. @ 21.5', high angle fracture. @ 22.4', highly weathered, fractured. @ 24.4', slightly to moderately weathered. @ 23.4', rust stained fractured. @ 29.1'-29.5', qu = 9,339 psi.													
20		Core 120"	Rec 120"																	
25																				
30																				

FILE: 0121-3070-03 [11/24/2006 2:14 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-416

Location: Sta. 480+94.2, 75.9 ft. RT of SR 823 CL

Date Drilled: 8/10/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 10.0' Water level at completion: None (prior to coring) 19.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - PL ----- LL Blows per foot - ○ 10 20 30 40				
30	682.5	Core 120"	Rec 120"	RQD 100%	R-2		MEDIUM HARD TO HARD BROWN SANDSTONE; very fine to fine grained, moderately weathered, micaceous, argillaceous. @ 30.8'-35.5', abundant to moderate argillaceous laminations. @ 36.5'-37.4', moderate to few laminations. @ 41.3'-41.6', 42.3'-42.9', few argillaceous laminations. @ 44.0', 44.2', low angle fractures. @ 44.7', 45.2', few argillaceous laminations.											
35																		
40		Core 120"	Rec 117"	RQD 98%	R-3													
45																		
50		Core 60"	Rec 60"	RQD 100%	R-4													
50.5	662.0						Bottom of Boring - 50.5'											
55																		
60																		

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-419

Location: Sta. 483+57.7, 141.2 ft. RT of SR 823 CL

Date Drilled: 8/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: Not reported	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0	656.6						DESCRIPTION Topsoil - 12" Loose to medium dense brown SANDY SILT (A-4a), trace gravel; dry. @ 3.5', contains sandstone fragments.											
1.0	655.6	3			1													
		3	18															
		9			2		Medium hard to hard gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, micaceous, massively bedded. @ 5.0' to 5.2', broken with typically low angle rust stained fractures. @ 5.5' to 5.7', broken with typically high angle rust stained fractures. @ 12.3' to 12.5', broken with typically low angle fractures.											
5.0	651.6	9	18															
		15																
		Core 60" Rec 60"			RQD 78%	R-1												
10		Core 120" Rec 120"			RQD 81%	R-2												
20.0	636.6						Bottom of Boring - 20.0'											
25																		
30																		

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-421

Location: Sta. 488+19.3, 4.0 ft. LT of SR 823 CL

Date Drilled: 7/9/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0.2	633.8						Topsoil - 2" Medium dense brown COARSE AND FINE SAND (A-3a); dry to damp. Medium dense brown SANDY SILT (A-4a), trace clay; damp.											
	633.6	5 6	15	1														
3.0	630.8	28 14 13	14	2														
5		6 5 6	14	3														
8.0	625.8						Soft brown and gray SANDSTONE; very fine to fine grained, moderately weathered, argillaceous, highly fractured. Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, micaceous, thickly bedded. @ 9.5', low angle, highly weathered fracture.											
9.0	624.8	50/5	5	4														
10		Core 60"	Rec 60"	RQD 90%	R-1													
14.0	619.8						Bottom of Boring - 14.0'											
15																		
20																		
25																		
30																		

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-424

Location: Sta. 491+05.2, on centerline SR 823

Date Drilled: 7/19/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.5' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	639.0																			
4		4				3.0	No Topsoil Very stiff brown SILT (A-4b), some clay, trace fine sand; contains sandstone fragments; damp.													
5		5 6	18	1																
5.5	633.5					2.0	Hard brown SANDY SILT (A-4a), little clay, trace gravel; damp.													
8.0		5 9 8	18	2					0	0	--	7	73	20						
8.0	631.0					4.5+	Severely weathered brown SANDSTONE argillaceous, micaceous.													
10.0		6 8 12	18	3					5	26	--	14	40	15						
10.0	629.0						Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, micaceous, medium bedded to massive, slightly fractured. @ 10.1', 10.2', 10.5' and 12.3', low angle fractures with iron staining.													
15		21 50/3	9	4																
20.0	619.0						@ 18.2'-18.6', iron stains.													
20.0		Core 36"	Rec 36"	RQD 92%	R-1															
20.0		Core 84"	Rec 84"	RQD 99%	R-2															
20.0	619.0						Bottom of Boring - 20.0'													
25																				
30																				

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-427

Location: Sta. 495+16.9, 1.1 ft. LT of SR 823 CL

Date Drilled: 7/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0.3	667.8						Topsoil - 3" Very stiff brown SANDY SILT (A-4a), trace clay, trace gravel; damp. @ 6.0', grayish brown.											
	667.5	4	18	1		3.0												
		4	18															
		10	18	2		4.0												
		15	18															
		8	18	3		3.5												
		8	18															
		3	18	4		4.0												
		6	18															
		7	18	5		4.0												
		9	18															
		11	18															
13.0	654.8						Severely weathered brown SANDSTONE.											
		43	8	6														
		50/2																
15.5	652.3						Soft to medium hard brown and gray SANDSTONE, argillaceous, highly weathered, moderately fractured. @ 15.8'-16.2', 18.6'-18.8', broken argillaceous interbeds. @ 16.5', 18.3', low angle fractures.											
		Core 54"	Rec 54"	RQD 100%	R-1													
20.0	647.8						Bottom of Boring - 20.0'											

FILE: 0121-3070-03 [11/24/2005 2:15 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-429

Location: Sta. 497+10.8, 5.6 ft. RT of SR 823 CL

Date Drilled: 7/15/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetro- meter (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 3.3' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	691.8																		
-0.5	691.3						Topsoil - 6"/1.9' soil removed before drilling												
		15 27 40	18			1	Very dense brown SANDY SILT (A-4a), little gravel, trace clay; contains sandstone fragments; damp.												○ 67
		15 27 30	18			2													○ 57
5																			
6.0	685.8						Soft brown BRECCIA/SANDSTONE/ SHALE, fine to medium grained, decomposed, broken.												
10		Core 84"	Rec 45"	RQD 54%															
15		Core 84"	Rec 68"	RQD 0%			@ 18.5'-18.6', 19.7'-19.8', hard SANDSTONE, possible cobbles or boulders.												
20.0	671.8						Bottom of Boring - 20.0'												
25																			
30																			

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-430

Location: Sta. 497+01.4, 86.8 ft. RT of SR 823 CL

Date Drilled: 7/16/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40		
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay			
0	685.5						No topsoil/12" soil removed before drilling Very stiff brown and red SANDY SILT (A-4a), little to some gravel; damp. Very stiff to hard brown and red SILT AND CLAY (A-6a), little fine to coarse sand, trace to little gravel; contains sandstone fragments; damp. Medium stiff brown and gray SANDY SILT (A-4a), trace clay; moist.									
		5 6 10	18	1		3.0										
		20 24 30	18	2		4.0										
5.5	680.0	7 10 17	18	3		4.5+										
		7 10 15	18	4		4.5+										
		9 10 12	18	5		3.75										
		6 10 11	18	6		2.75										
15.5	670.0	4 5 5	18	7		0.5										
		4 5 8	18	8		0.75										
20.0	665.5						Bottom of Boring - 20.0'									

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-432

Location: Sta. 498+11.4, 111.1 ft. LT of SR 823 CL

Date Drilled: 7/15/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 4.9' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay								
0	738.4						<p><i>DESCRIPTION</i></p> <p>Topsoil - 7"/2.1' soil removed before drilling</p> <p>Dense brown SANDY SILT (A-4a), little gravel; contains sandstone fragments; damp.</p>														
0.6	737.8	14 14 17	18	14		1															
4.5	733.9	14 17 50/3	18	14		1		Severely weathered brown SANDSTONE.													
6.0	732.4						<p>Soft to medium hard light brown SANDSTONE, argillaceous, highly weathered to decomposed, broken, contains numerous clay coated and rust stained low angle fractures, broken.</p> <p>@ 6.4'-6.8', high-angle clay filled fracture.</p> <p>@ 9.3'-9.8', high-angle clay filled fracture.</p> <p>@ 8.7'-10.3', qu = 5,466 psi, SDI = 73.6%.</p> <p>@ 10.2'-11.6', high-angle clay filled fracture.</p> <p>@ 12.4'-12.7', high-angle healed fracture.</p> <p>@ 13.0'-13.5', very argillaceous.</p> <p>@ 13.0'-13.5', 14.0'-15.1', 16.5'-16.6', 17.3'-17.4', decomposed broken zones.</p> <p>@ 14.6'-17.8', partially clay filled near vertical fracture.</p>														
10		Core 84"	Rec 84"	RQD 40%	R-1	*36															
18.0	720.4	Core 120"	Rec 116"	RQD 48%	R-2	*162	<p>Hard gray SANDSTONE, very fine to fine grained, moderately weathered, thin bedded to medium bedded, contains turbidity bedding, moderately to slightly fractured.</p> <p>@ 18.0'-18.6', brown and gray interbedded.</p> <p>@ 18.2' and 18.4', low angle fracture containing argillaceous laminations.</p> <p>@ 18.9', low angle fracture with rust staining.</p> <p>@ 24.5' and 28.7', low angle highly weathered fracture containing argillaceous laminations.</p> <p>@ 26.5'-27.0', qu = 8,267 psi.</p>														
25		Core 120"	Rec 120"	RQD 100%	R-3	*414															
30																					

FILE: 0121-3070-03 [11/24/2005 2:16 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-432

Location: Sta. 498+11.4, 111.1 ft. LT of SR 823 CL

Date Drilled: 7/15/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 4.9' (Includes drilling water)	GRADATION					STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
30	708.4						DESCRIPTION Hard gray SANDSTONE, very fine to fine grained, moderately weathered, thin bedded to medium bedded, contains turbidity bedding, moderately to slightly fractured. @ 30.1', 30.5', low angle fracture containing argillaceous lamination. @ 33.4'-34.9', rust stained, near vertical fracture. @ 35.5'-36.0', iron inclusions.													
35		Core 84"	Rec 84"	RQD 77%	R-4	*320														
40.0	698.4							Bottom of Boring - 40.0'												
45																				
50																				
55																				
60																				

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-433

Location: Sta. 498+14.4, 21.4 ft. LT of SR 823 CL

Date Drilled: 7/15/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 10.0' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - PL ————— LL Blows per foot - ○						
0	725.9																			
0.5	725.4						Topsoil - 6"/1.9' soil removed before drilling													
		15 23 17	18			1	Dense to very dense brown and gray SANDY SILT (A-4a), trace gravel; damp.													
4.0	721.9	23 37 50/2	14			2	Severely weathered brown SANDSTONE													
6.0	719.9						Medium hard light brown SANDSTONE, fine to medium grained, argillaceous, highly weathered to decomposed, broken; contains numerous silty clay filled, rust stained, low angled fractures. @ 7.4'-7.6', high angle rust stained fracture. @ 8.2'-8.5', decomposed and very argillaceous.													
10		Core 84"	Rec 84"	RQD 75%	R-1	*112	@ 11.6', low angle fracture. @ 12.0', 12.1', rust stained low angle fracture. @ 12.2'-12.4', brown and gray.													
15		Core 120"	Rec 120"	RQD 73%	R-2	*512	Medium hard to hard gray SANDSTONE, very fine to fine grained, laminated to thin bedded, moderately weathered, contains very small argillaceous clasts, highly fractured. @ 13.3', very thin decomposed argillaceous zone. @ 13.7', 13.9' and 14.0', near horizontal rust stained fractures. @ 15.5'-16.7', contains contains soft to medium hard brown argillaceous interbeds, highly weathered to decomposed.													
20							@ 21.6', 22.1', 22.3' and 22.5', near horizontal rust stained fractures. @ 22.2'-22.5', high angle partially healed fracture with rust staining.													
25.0	700.9	Core 24"	Rec 24"	RQD 92%	R-3	*387	@ 22.7' and 23.8', horizontal partially clay filled fractures.													
							Bottom of Boring - 25.0'													

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-434

Location: Sta. 498+47.1, 133.5 ft. RT of SR 823 CL

Date Drilled: 7/15/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 7.9' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	10	20	30	40			
0	709.3						<p>DESCRIPTION</p> <p>Topsoil - 5"/1.4' soil removed before drilling</p> <p>Medium dense reddish brown SANDY SILT (A-4a), little gravel; contains sandstone fragments; damp.</p> <p>Severely weathered brown SANDSTONE.</p>													
0.4	708.9	7	7	18	1															
3.0	706.3	17	35	50/4	16	2														
6.0	703.3						<p>Medium hard light brown and brown SANDSTONE, fine to medium grained, argillaceous, highly weathered, broken.</p> <p>@ 6.3', 6.5', 8.0', horizontal fractures.</p> <p>@ 6.7'-7.7', 8.2'-8.9', broken with clay layers, decomposed.</p> <p>@ 9.1', 10.0', and 10.3', partially clay filled low angle fracture.</p> <p>@ 11.0'-11.2', high angle, clay filled fracture.</p> <p>@ 11.9', low angle clay filled fracture.</p> <p>@ 12.3'-13.3', broken with clay layers.</p> <p>@ 13.6 becomes grayish brown.</p> <p>@ 14.6'-14.9', partially healed rust stained high angle fracture.</p>													
10		Core 84"	Rec 84"	RQD 39%	R-1	*193														
15.9	693.4						<p>Medium hard to hard gray SANDSTONE, very fine to fine grained, moderately weathered, thin bedded to laminated, contains very small argillaceous clast.</p> <p>@ 16.3'-16.6' and 17.8'-18.0', rust stained horizontal fracture.</p> <p>@ 18.5'-18.7', partially healed rust stained high angle fracture.</p>													
20.0	689.3	Core 84"	Rec 84"	RQD 71%	R-2	*90														
20.0							Bottom of Boring - 20.0'													

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-435 Location: Sta. 501+08.3, 201.1 ft. LT of SR 823 CL Date Drilled: 7/15/04 to 7/19/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (lost water circulation)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40								
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay									
0-1	847.7 847.6						Topsoil - 1"															
		5 22 15	18			1	Soft to medium hard brown SANDSTONE; decomposed, argillaceous.															
		22 50/3	8			2																
		41 50/2	1			3																
		50/5	4			4	@ 8.0', light brown.															
		50/4	4			5																
13.0	834.7	Core 12"	Rec 12"	RQD 67%	R-1	*181	Medium hard gray SANDSTONE, fine grained, thin to thick bedded, moderately to highly weathered, moderately fractured, contains occasional small argillaceous clasts, contains weathered rust stained zones. @ 16.0'-16.1', high-angle fracture. @ 17.1'-17.6', qu = 5,833 psi.															
		Core 120"	Rec 120"	RQD 87%	R-2	*240	@ 19.7', 0.1-ft. ironstone band, contains argillaceous laminations. @ 20.8', high angle clay filled fracture.															
21.9	825.8	Core 120"	Rec 120"	RQD 90%	R-3	*308	Medium hard to hard gray SANDSTONE, very fine to fine grained, medium to thick bedded, moderately weathered, high to slightly fractured, contains occasional small argillaceous clasts. @ 25.2'-25.4' & 26.4'-27.0', high angle clean fracture. @ 26.2', low angle fracture, contains argillaceous laminations. @ 29.7', low angle fracture.															

FILE: 0121-3070-03 [11/24/2006 2:15 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-435

Location: Sta. 501+08.3, 201.1 ft. LT of SR 823 CL

Date Drilled: 7/15/04 to 7/19/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % -							
							Water seepage at: None Water level at completion: None (lost water circulation)														
30	817.7																				
35																					
40		Core 120"	Rec 120"	RQD 89%	R-4	*366	Medium hard to hard gray SANDSTONE, very fine to fine grained, medium to thick bedded, moderately weathered, high to slightly fractured, contains occasional small argillaceous clasts. @ 30.0'-30.8', brown highly weathered with turbidite beds, rust stained zone. @ 30.2'-30.4', horizontal infilled rust stained fracture. @ 36.1', horizontal spotty clay filled fracture. @ 36.8', contains fossils with pyrite. @ 39.6'-39.9', qu = 9,802 psi.														
45							@ 44.0'-44.2', hard light gray calcareous zone, vuggy, pyritic, moderately weathered. @ 46.6'-51.1', brownish gray bed.														
50		Core 120"	Rec 120"	RQD 98%	R-5	*433	@ 51.4' and 51.6', low angle smooth clean fracture, contains argillaceous laminations. @ 55.8'-60.4', brownish gray. @ 56.6', low angle fracture.														
55							@ 59.4'-60.0', high angle fracture.														
60		Core 120"	Rec 120"	RQD 95%	R-6	*413															

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-436 Location: Sta. 501+35.7, 85.8 ft. LT of SR 823 CL Date Drilled: 7/13/04 to 7/14/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 55.5' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)							
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40							
0.3	827.2						Topsoil - 4"/12" soil removed before drilling Soft to medium hard brown SANDSTONE; decomposed.														
	826.9	50/5	4	1																	
		50/5	2	2																	
5							Medium hard brown SANDSTONE, very fine to fine grained, thickly bedded, highly weathered, broken. @ 6.0'-7.0', very broken with low angle fractures and rust staining. @ 8.7', 9.2' & 9.6', low angle rust stained and filled fractures. @ 10.0', high angle clay filled fracture. @ 10.9', low angle partially clay filled fracture. @ 11.6'-12.1', very broken with clay seams. @ 12.9', low angle partially clay filled fracture. @ 13.3', low angle clean smooth fracture. @ 13.6', becomes brown and gray. @ 14.6', low angle fracture with highly weathered to decomposed zone. @ 14.9'-15.6', highly broken with low angle and high angle fractures with rust staining. @ 16.2'-16.7', high angle clean rough fracture. @ 17.5', low angle clay filled fracture. @ 19.6', low angle rust stained fracture. @ 27.5', low angle rust stained clay coated fracture. @ 29.3', low angle fracture.														
6.0	821.2	Core 48"	Rec 45"	RQD 52%	R-1	*47															
10																					
15		Core 120"	Rec 120"	RQD 76%	R-2	*180															
20																					
25		Core 120"	Rec 120"	RQD 76%	R-3	*295															
30																					

FILE: 0121-3070-03 [11/24/2006 2:15 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-436

Location: Sta. 501+35.7, 85.8 ft. LT of SR 823 CL

Date Drilled: 7/13/04 to 7/14/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N)					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % -		Blows per foot -			
							Water seepage at: None Water level at completion: None (Prior to coring) 55.5' (Includes drilling water)												
30	797.2																		
35		Core 120"	Rec 120"	RQD 95%	R-4	*249	Medium hard brown and gray SANDSTONE, very fine to fine grained, thin to thickly bedded, moderately weathered, contains turbidite beds, laminated to massive. @ 31.3'-32.2', high angle clay coated rough fracture.												
40							@ 41.7' to 41.8', argillaceous zone. @ 42.8' to 42.9', argillaceous zone.												
45		Core 120"	Rec 120"	RQD 98%	R-5	*384	@ 45.0', 45.1' & 45.5', low angle clay coated fracture with rust staining. @ 45.3', rust stained, low angle fracture. @ 45.3'-45.4', argillaceous zone. @ 46.3'-46.4', argillaceous zone. @ 46.7', low angle fracture with rust staining. @ 48.4'-48.5', argillaceous zone. @ 48.5', low angle fracture with rust staining. @ 49.7', low angle fracture with rust staining.												
50																			
55		Core 120"	Rec 120"	RQD 97%	R-6	*269	@ 56.5' and 56.9', argillaceous laminations. @ 58.3', thin argillaceous laminations.												
60																			

FILE: 0121-3070-03 [11/24/2005 2:16 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-437

Location: Sta. 501+22.1, 86.1 ft. RT of SR 823 CL

Date Drilled: 7/12/04 to 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 14.8' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40						
0	771.8						No topsoil/1.5' soil removed before drilling Very stiff brown SILT AND CLAY (A-6a), trace fine to coarse sand, trace gravel; contains sandstone fragments; damp.													
		5	18	1																
		5	16	2		2.5														
5		12	16																	
		9	16	2																
		6	18	3		3.5														
		2	18																	
8.0	763.8						Soft to medium hard brown SANDSTONE; very fine to fine grained, decomposed to highly weathered, argillaceous.													
		31	10	4																
		50/4																		
10																				
		25	11	5																
		46	11																	
		50/4																		
15																				
		18	7	6																
		50/3																		
17.4	754.4						@ 16.6', rust stained, low angle fracture. @ 17.2'-17.3', rust stained, high angle fracture.													
		50/3	2	7																
20							Medium hard to hard gray SANDSTONE; very fine to fine grained, highly weathered, argillaceous, thinly bedded to medium bedded, moderately fractured.													
		Core 42"	Rec 42"	RQD 79%	R-1	*278														
25							@ 19.9'-20.4', light brown colored. @ 20.4', pyritic. @ 21.3', moderately weathered. @ 21.3'-24.8', light brown colored. @ 21.8'-22.0', 22.1'-22.6', 23.4', 24.1', high angle rust stained fractures.													
		Core 120"	Rec 118"	RQD 78%	R-2	*902														
30							@ 28.7'-30.0', contains few argillaceous laminations.													

FILE: 0121-3070-03 [11/24/2006 2:15 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-437

Location: Sta. 501+22.1, 86.1 ft. RT of SR 823 CL

Date Drilled: 7/12/04 to 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / *Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 14.8' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ——— LL Blows per foot - ○ 10 20 30 40			
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay				
30	741.8						MEDIUM HARD TO HARD GRAY SANDSTONE; very fine to fine grained, moderately to slightly weathered, argillaceous, thinly bedded to medium bedded, moderately fractured, contains few argillaceous laminations. @ 31.1', 31.3', 31.4', 31.9', 32.8', low angle rust stained fracture. @ 35.8'-37.2', near vertical fracture with iron filling. @ 36.5', 36.8', 40.6', 41.1', contains argillaceous laminae @ 38.3'-38.9', near vertical fracture with iron filling.										
35		Core 120"	Rec 118"	RQD 75%	R-3	*1143											
39.2	732.6						Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, thinly bedded to medium bedded, slightly fractured, contains turbidite beds, slightly fractured. @ 42.4', contains argillaceous lamination. @ 43.0', 47.1', 49.1', contains argillaceous laminae. @ 45.5'-45.7', contains argillaceous laminae. @ 45.9'-46.5', qu = 9,741 psi. @ 46.5'-46.8', argillaceous laminae. @ 48.9' and 49.1', filled fracture. @ 49.1'-49.2', low angle fracture with argillaceous lamination. @ 50.0'-51.8', contains few argillaceous laminae.										
40		Core 120"	Rec 120"	RQD 97%	R-4	*1262											
45		Core 120"	Rec 120"	RQD 97%	R-4	*1262											
50		Core 120"	Rec 120"	RQD 100%	R-5	*1038											
55		Core 120"	Rec 120"	RQD 100%	R-5	*1038											
60.0	711.8						@ 53.6'-53.7', argillaceous laminations. @ 58.6', argillaceous laminations. Bottom of Boring - 60.0'										

FILE: 0121-3070-03 [11/24/2006 2:15 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-437

Location: Sta. 501+22.1, 86.1 ft. RT of SR 823 CL

Date Drilled: 7/12/04 to 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 14.8' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○		
							DESCRIPTION											
60	711.8																	
65																		
70																		
75																		
80																		
85																		
90																		

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-439 Location: Sta. 504+61.7, 91.4 ft. RT of SR 823 CL Date Drilled: 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 6.8' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay					
0	696.6						Topsoil - 5" Loose to medium dense brown SANDY SILT (A-4a), trace gravel; contains sandstone fragments; damp.											
0.5	696.1	4	4	18		1												
4.5	692.1	7	10	50/2	14	2	Severely weathered brown SANDSTONE.											
5																		
6.0	690.6						Medium hard brown SANDSTONE; very fine to fine grained, moderately weathered, moderately fractured to slightly fractured. @ 6.6', low angle rust stained fracture. @ 6.9', low angle fracture.											
10		Core 84"	Rec 84"	RQD 100%	R-1	*408												
15		Core 84"	Rec 84"	RQD 100%	R-2	*381	@ 15.4'-20.0', contains moderate argillaceous laminae.											
20.0	676.6						Bottom of Boring - 20.0'											
25																		
30																		

FILE: 0121-3070-03 [11/24/2006 2:17 PM]

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-440 Location: Sta. 504+45.8, 141.8 ft. RT of SR 823 CL Date Drilled: 7/14/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 5.6' (Includes drilling water)	GRADATION					STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ←————→ LL Blows per foot - ○ 10 20 30 40	
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt		% Clay
DESCRIPTION														
0.2	695.2						Topsoil - 2"							
0.2 - 3.0	695.0 - 692.2	3 4	18	1		2.0	Stiff to very stiff dark brown SILT (A-4b), trace fine to coarse sand; damp.							
3.0 - 6.0	692.2 - 689.2	6 7 14	18	2			Soft grayish brown SANDSTONE; very fine grained, decomposed, argillaceous.							
6.0 - 13.5	689.2 - 681.7	Core 90"	Rec 90"	RQD 92%	R-1		Medium hard gray SANDSTONE, fine to medium grained, contains argillaceous clasts, highly weathered, broken with clay seams. @ 6.3', 7.1' iron stained. @ 6.8', iron stained fracture. @ 7.1'-8.1', contains argillaceous laminae. @ 8.5'-8.7', high angle fracture, iron stained in fracture, rock partly decomposed. @ 10.1'-13.5' contains few argillaceous interbeds. @ 10.6', moderately weathered.							
13.5	681.7						Bottom of Boring - 13.5'							

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-442

Location: Sta. 505+27.7, 9.2 ft. LT of SR 823 CL

Date Drilled: 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (prior to coring) 4.8' (includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay						
0	711.5						No topsoil Medium dense brown GRAVEL WITH SAND AND SILT (A-2-4), trace clay; contains sandstone fragments; damp.												
		49 10 6	18			1													
4.0	707.5	7 50/3	9			2	Severely weathered brown SANDSTONE.												
7.5	704.0						Soft to medium hard brown SANDSTONE; fine to medium grained, highly weathered, argillaceous, broken. @ 7.5'-7.6', 8.7', 9.6', 10.2, low angle fractures. @ 8.1'-8.2', 9.5'-9.7', high angle fractures.												
10.2	701.3	Core 84"	Rec 84"	RQD 76%	R-1			Medium hard to hard gray SANDSTONE, moderately to slightly fractured, very fine to fine grained, moderately weathered, moderately to slightly fractured, contains medium to massive turbidite beds.											
15		Core 84"	Rec 84"	RQD 100%	R-2		@ 16.9', fracture.												
20.0	691.5						Bottom of Boring - 20.0'												
25																			
30																			

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-444

Location: Sta. 508+34.4, 121.0 ft. LT of SR 823 CL

Date Drilled: 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 5.7' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	782.4																			
0.7	781.7	8					Topsoil - 8"/2.0' soil removed													
		12	18			1	Medium dense reddish brown SANDY SILT (A-4a), little gravel; damp.													
		15																		
3.0	779.4	20				2	Severely weathered brown SANDSTONE.													
		24	18																	
		28																		
5																				
		26				3														
		50/5	11																	
7.0	775.4						Soft to medium hard brown and gray SANDSTONE; very fine to fine grained, highly weathered to decomposed, argillaceous, micaceous, thinly bedded to thickly bedded, broken.													
10		Core 72"	Rec 72"	RQD 38%	R-1	*18	@ 10.8'-11.8', vertical fracture.													
14.2	768.2						Medium hard gray CLAY SHALE interbedded with SANDSTONE, highly weathered, moderately fractured, laminated to thinly bedded.													
15		Core 120"	Rec 120"	RQD 93%	R-2	*324														
20.3	762.1						Medium hard to hard gray SANDSTONE; very fine to fine grained, highly weathered, micaceous, thinly bedded to thickly bedded, high to moderately fractured. @ 20.0', 2 low angle clay filled fractures. @ 21.6' and 22.3', low angle clay filled fracture. @ 23.2'-23.5', high angle partially healed fracture with calcite. @ 24.4', low angle fracture. @ 24.6', pyritic with occasional fossils.													
25		Core 120"	Rec 120"	RQD 87%	R-3	*523	@ 27.9', 28.4', low angle fracture.													
30																				

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring R-444

Location: Sta. 508+34.4, 121.0 ft. LT of SR 823 CL

Date Drilled: 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 5.7' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N)				
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○		
							DESCRIPTION											
30	752.4																	
35		Core 84"	Rec 84"	RQD 100%	R-4	*563	Medium hard to hard gray SANDSTONE; very fine to fine grained, highly weathered, micaceous, thinly bedded to thickly bedded, high to moderately fractured. @ 32.0'-32.5', high angle fracture.											
40.0	742.4						Bottom of Boring - 40.0'											
45																		
50																		
55																		
60																		

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring R-445 Location: Sta. 508+42.3, 128.2 ft. RT of SR 823 CL Date Drilled: 7/13/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetro- meter (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: None (Prior to coring) 11.9' (Includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	779.8						Topsoil - 4"/1.1' soil removed before drilling Medium dense brown SANDY SILT (A-4a), little gravel; damp.													
0.3	779.5	7	18	1																
3.0	776.8	50/3	3	2			Severely weathered brown SANDSTONE.													50+
6.0	773.8						Soft brown and gray SANDSTONE; fine to medium grained, highly weathered to decomposed, argillaceous, micaceous, laminated to thickly bedded, broken. @ 7.8', thin clay seam. @ 8.8' and 9.8', thin clay seam with rock fragments. @ 10.4' and 11.9', clay seam with rock fragments, very fine to medium grained interbedded sandstone.													
10		Core 84"	Rec 84"	RQD 88%	R-1	*167														
12.0	767.8						Soft gray and brown CLAY SHALE; decomposed to highly weathered, arenaceous, micaceous, laminated to thinly bedded, broken.													
15		Core 120"	Rec 120"	RQD 79%	R-2	*68														
19.2	760.6						Medium hard brown and gray SANDSTONE; very fine to fine grained, moderately to highly weathered, pyritic, thinly bedded to medium bedded, broken to highly fractured, contains thin argillaceous beds, contains turbidite beds. @ 21.5', gray. @ 21.7'-22.3', argillaceous. @ 23.5', iron stained low angle fracture. @ 24.6', clay filled fracture. @ 25.0'-27.8', pyritic inclusions. @ 26.0', 26.2', 26.6' and 27.3', low angle clay filled fractures.													
20																				
25		Core 120"	Rec 113"	RQD 88%	R-3	*201	@ 29.0'-30.8', brown, very broken with core loss.													
30																				

FILE: 0121-3070-03 [11/24/2006 2:17 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring TR-16

Location: Sta. 486+12.4, 32.3 ft. LT of SR 823 CL

Date Drilled: 7/9/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 6.0' Water level at completion: 6.5'	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40	
				Drive	PBSS / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
0.2	631.9						Topsoil - 2" Medium stiff brown SANDY SILT (A-4a); moist.								
	631.7	2			1	1.0									
		3	2	16											
		1			2	0.75									
5		1	1	15											
		4			3	--	@ 6.0' to 7.4', contains rock fragments.								
		10		12											
		50/5													
8.5	623.4						Medium hard to hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, argillaceous, massively bedded, slightly fractured.								
10															
		Core 120"	Rec 118"		RQD 85%	R-1									
15							@ 17.0', contains few argillaceous laminations.								
18.5	613.4						Bottom of Boring - 18.5'								
20															
25															
30															

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring TR-17 Location: Sta. 485+26.9, 24.3 ft. RT of SR 823 CL Date Drilled: 2/23/2005

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	631.7																			
0.4	631.3						Water seepage at: 6.3'-7.0' Water level at completion: 1.6' (inside hollowstem augers, includes drilling water)													
		6 8 10	18			1	Topsoil - 5"													
3.0	628.7						Medium dense brown SILT (A-4b), little fine to coarse sand, trace clay; damp.													
		6 4 5	18			2	Loose brown GRAVEL WITH SAND AND SILT (A-2-4); damp.													
5	626.2						Very dense brown SANDY SILT (A-4a); wet.													
5.5	625.4	3				3A	Severely weathered gray SANDSTONE.													
6.3	624.7	50/5	11			3B	Medium hard brown and gray SANDSTONE; fine grained, moderately weathered, slightly micaceous, slightly fractured. @ 7.3'-7.4', very soft, highly weathered. @ 8.5', irregular fracture. @ 8.7', gray.													
7.0	624.7																			
10		Core 120"	Rec 120"			RQD 83%	@ 16.0', 1" soft, weathered zone.													
15						R-1														
17.0	614.7						Hard brown and gray SANDSTONE; fine grained, slightly weathered, slightly micaceous, slightly fractured.													
20		Core 120"	Rec 120"			RQD 97%	@ 22.8'-23.0', very soft, highly weathered siltstone seam. @ 23.0'-23.2', siltstone seam.													
25						R-2														
27.0	604.7						Bottom of Boring - 27.0'													
30																				

FILE: 0121-3070-03 [11/23/2005 2:18 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring TR-18

Location: Sta. 484+38.6, 39.0 ft. LT of SR 823 CL

Date Drilled: 8/17/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: None Water level at completion: 9.4' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40						
0	631.3																			
1.0	630.3	2 3 3	18	1			Topsoil - 12"	13	7	-	9	58	13							
3.0	628.3	3 4 4	18	2			Medium stiff brown SILT (A-4b), little clay, little fine to coarse sand, little gravel; contains roots; dry to damp.	0	3	-	40	45	12							
5		6 7 50/3	12	3			Loose brown SANDY SILT (A-4a), little clay, trace to little gravel; damp.	11	20	-	28	31	10							
7.3	624.0						Hard gray SANDSTONE; very fine to fine grained, slightly weathered, argillaceous, micaceous, slightly to moderately fractured. @ 7.3'-7.6', broken. @ 7.3'-7.8', 8.0', 8.6'-8.8', brown, rust-stained fractures. @ 7.3'-7.8', vertical fracture.													
10		Core 84"	Rec 84"	RQD 88%	R-1															
15		Core 72"	Rec 71"	RQD 94%	R-2															
20.3	611.0						Bottom of Boring - 20.3'													

FILE: 0121-3070-03 [11/24/2006 2:18 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring TR-19

Location: Sta. 483+69.8, 46.5 ft. RT of SR 823 CL

Date Drilled: 8/16/04 to 8/17/04

Depth (ft)	Elev. (ft)	Blows per 6"		Recovery (in)	Sample No.		Hand Penetrometer (tsf) / Point-Load Strength (psi)	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40					
		Drive	Press / Core		WATER OBSERVATIONS:	% Aggregate			% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	633.0							Water seepage at: None Water level at completion: 16.3' (includes drilling water)												
1.0	632.0	3						Topsoil - 12"												
		7	7	18			1	Medium dense brown SANDY SILT (A-4a), trace gravel, trace clay; contains sandstone fragments; damp.												
		4	7	9	18		2													
5		4	5	6	18		3													
8.7	624.3	50/2		2			4	Medium hard to hard gray SANDSTONE; very fine to fine grained, slightly to moderately weathered, argillaceous, micaceous, massively bedded, slightly fractured. @ 9.2'-9.4', decomposed. @ 8.8'-9.0', brown. @ 13.1'-13.3', vertical fracture. @ 13.9'-14.0', vertical fracture. @ 15.5', unfractured to slightly fractured. @ 14.7'-15.5', broken zone. @ 15.4'-15.5', clay filled fracture.												
10		Core 30"		Rec 30"			RQD 57%		R-1											
15		Core 108"		Rec 108"			RQD 70%		R-2											
20.2	612.8							Bottom of Boring - 20.2'												
25																				
30																				

Client: TranSystems, Inc. Project: SCI-823-0.00 Job No. 0121-3070.03

LOG OF: Boring TR-21 Location: Sta. 443+67.0, 46.5 ft. LT of SR 823 CL Date Drilled: 8/3/04

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: 0.0' Water level at completion: 0.0' (includes drilling water)	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural Moisture Content, % - ●		Blows per foot - ○				
							DESCRIPTION													
0	639.0						Gray GRAVEL (A-1-a); wet. (Auger sample - boring drilled in stream bed)													
1.5	637.5						Medium hard to hard gray SANDSTONE; very fine to fine grained, slightly weathered, micaceous, argillaceous, massively bedded, slightly fractured. @ 1.5'-3.9', brown, highly weathered, highly fractured to broken. @ 3.3'-3.4', clay filled fracture.													
5		Core 114"	Rec 114"	RQD 70%	R-1															
15		Core 108"	Rec 108"	RQD 93%	R-2															
20.0	619.0						Bottom of Boring - 20.0'													
25																				
30																				

FILE: 0121-3070-03 [11/24/2006 2:18 PM]

Client: TranSystems, Inc.

Project: SCI-823-0.00

Job No. 0121-3070.03

LOG OF: Boring TR-22

Location: Sta. 442+46.9, 51.5 ft. RT of SR 823 CL

Date Drilled: 2/24/2005

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery (in)	Sample No.		Hand Penetrometer (tsf) / * Point-Load Strength (psi)	WATER OBSERVATIONS: Water seepage at: none Water level at completion: 4.5' (inside hollowstem augers, includes drilling water)	GRADATION						STANDARD PENETRATION (N) Natural Moisture Content, % - ● PL ————— LL Blows per foot - ○ 10 20 30 40
				Drive	Press / Core			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	
0	636.2						<p>DESCRIPTION</p> <p>Topsoil - 8"</p> <p>Stiff brown SANDY SILT (A-4a), trace gravel; organic; moist.</p> <p>Very dense brown SANDY SILT (A-4a), trace gravel; organic; moist.</p> <p>Severely weathered brown SANDSTONE.</p> <p>Soft brown SANDSTONE; fine grained, moderately weathered, slightly micaceous, moderately fractured. @ 5.2'-5.7', 7.1'-7.3', 8.7'-8.9' very soft, highly weathered. @ 6.1', gray, medium hard.</p> <p>@ 10.9'-11.0', iron stained horizontal fractures. @ 12.0'-12.8', siltstone.</p> <p>Hard gray SANDSTONE; fine grained, slightly weathered, slightly micaceous, slightly fractured. @ 14.7'-15.3', very soft gray and brown SILTSTONE, highly weathered.</p> <p>@ 19.3'-19.4', irregular vertical fracture. @ 19.6', 1/2" clay filled fracture.</p> <p>@ 23.2'-23.5', siltstone.</p> <p>Bottom of Boring - 24.0'</p>							
0.7	635.5	3		1	1.25									
		6	7	18										
2.8	633.4	26				2A								
3.5	632.7	50/4	10			2B								
4.0	632.2													
5														
10		Core 120"	Rec 120"	RQD 84%	R-1									
14.0	622.2													
15														
20		Core 120"	Rec 120"	RQD 96%	R-2									
24.0	612.2													
25														
30														

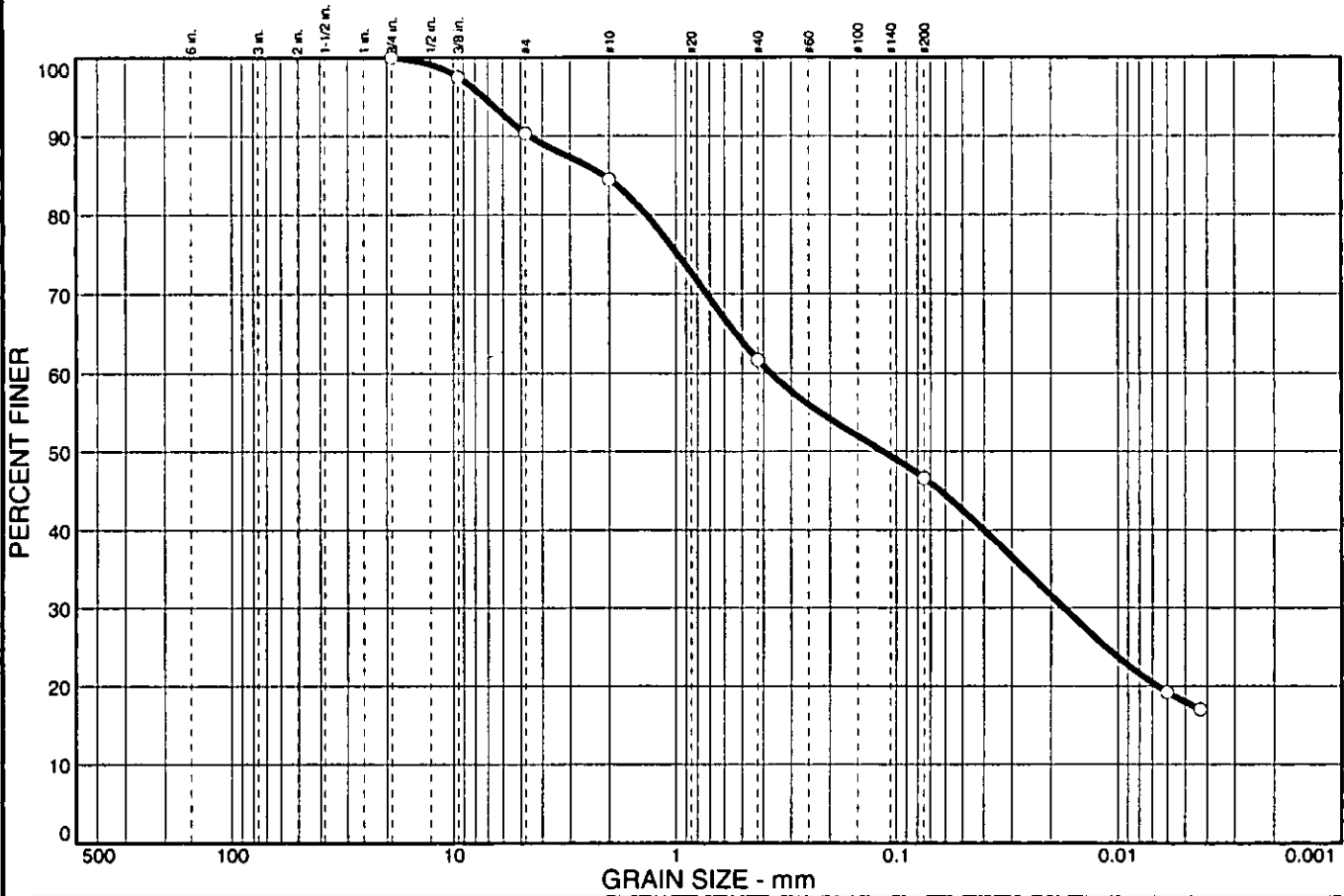
FILE: 0121-3070-03 [11/24/2005 2:13 PM]



APPENDIX B

Laboratory Test Results

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	9.7	5.8	22.8	15.2	28.5	18.0

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	97.5		
#4	90.3		
#10	84.5		
#40	61.7		
#200	46.5		

Soil Description

Clayey sand

Atterberg Limits

PL= 18 LL= 26 PI= 8

Coefficients

D₈₅= 2.12 D₆₀= 0.372 D₅₀= 0.116
 D₃₀= 0.0176 D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= SC AASHTO= A-4(1)

Remarks

Moisture Content= 12.7%
 F.M.=0.12

* (no specification provided)

Sample No.: 1
 Location:

Source of Sample: B-6

Date: 7/14/06
 Elev/Depth: 0.5

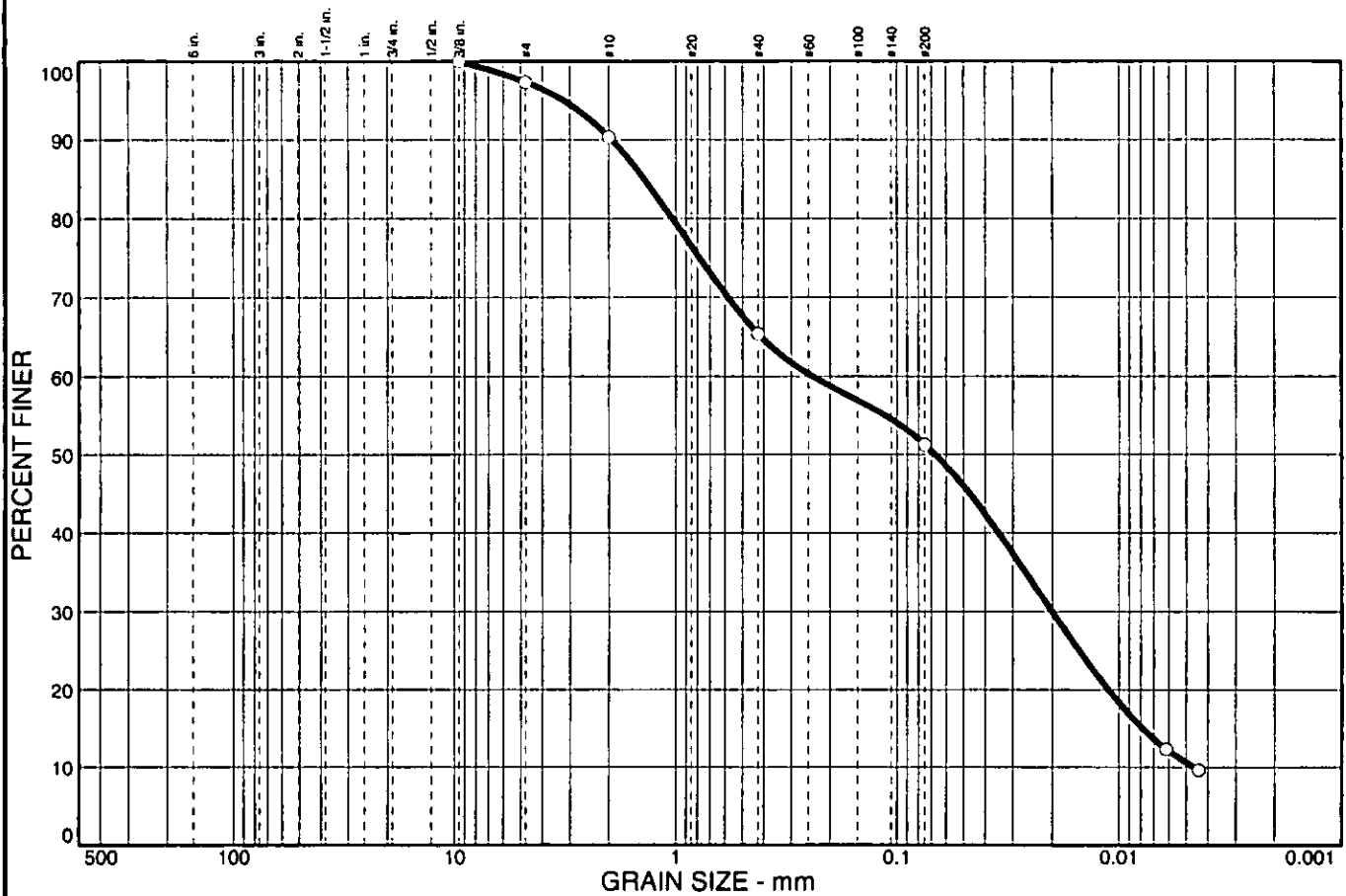


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	2.7	7.0	24.9	14.2	40.6	10.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	97.3		
#10	90.3		
#40	65.4		
#200	51.2		

Soil Description

Sandy silt

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 1.39 D₆₀= 0.241 D₅₀= 0.0677
 D₃₀= 0.0201 D₁₅= 0.0079 D₁₀= 0.0046
 C_u= 51.92 C_c= 0.36

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 11.7%
F.M.=0.03

* (no specification provided)

Sample No.: 2
Location:

Source of Sample: B-6

Date: 7/14/06
Elev./Depth: 3.5

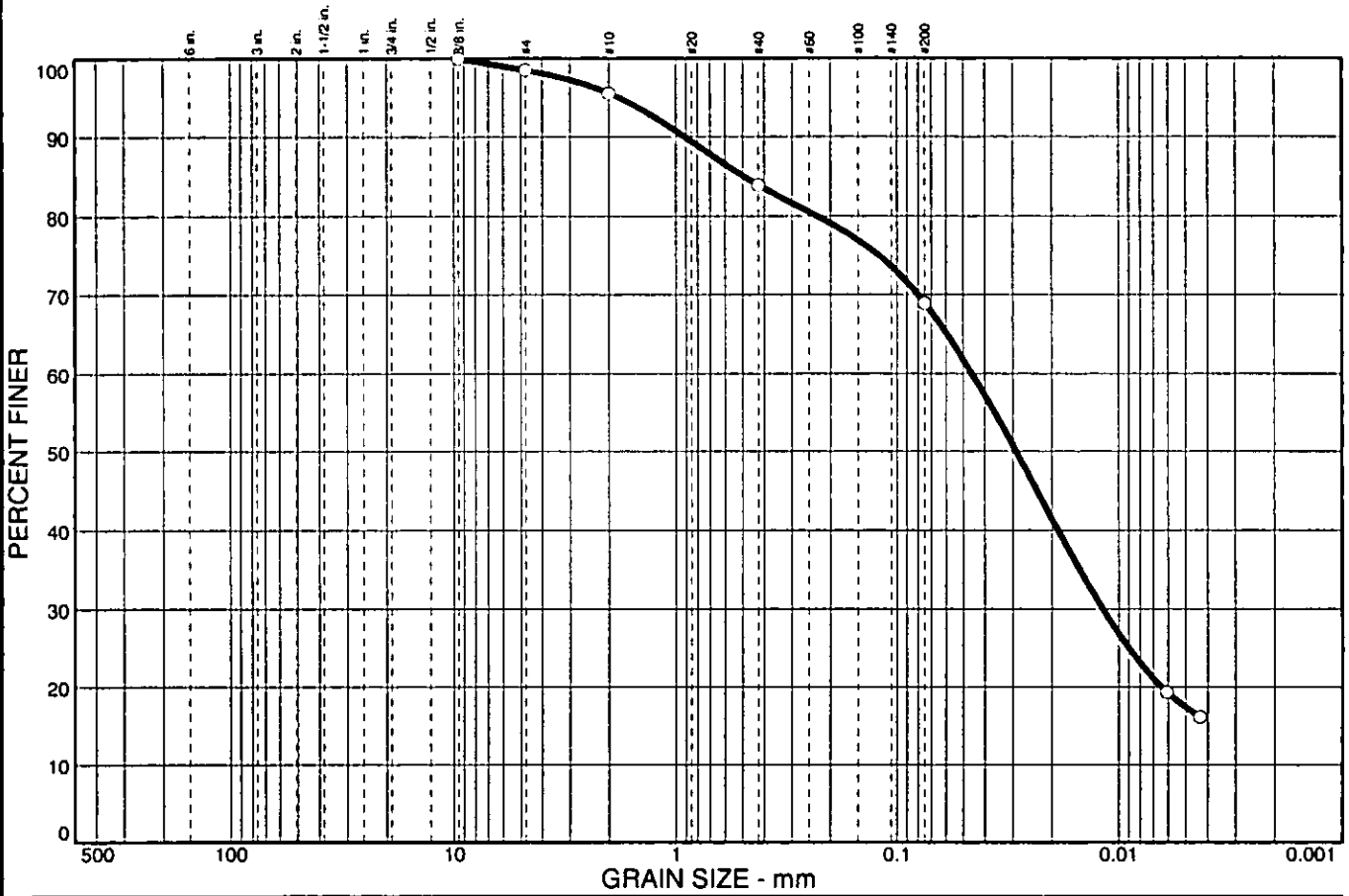


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	1.5	3.0	11.6	15.0	51.5	17.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	98.5		
#10	95.5		
#40	83.9		
#200	68.9		

Soil Description

Sandy silty clay

Atterberg Limits

PL= 18 LL= 22 PI= 4

Coefficients

D₈₅= 0.494 D₆₀= 0.0460 D₅₀= 0.0292
D₃₀= 0.0118 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(0)

Remarks

Moisture Content= 13.4%
F.M.=0.01

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: B-8

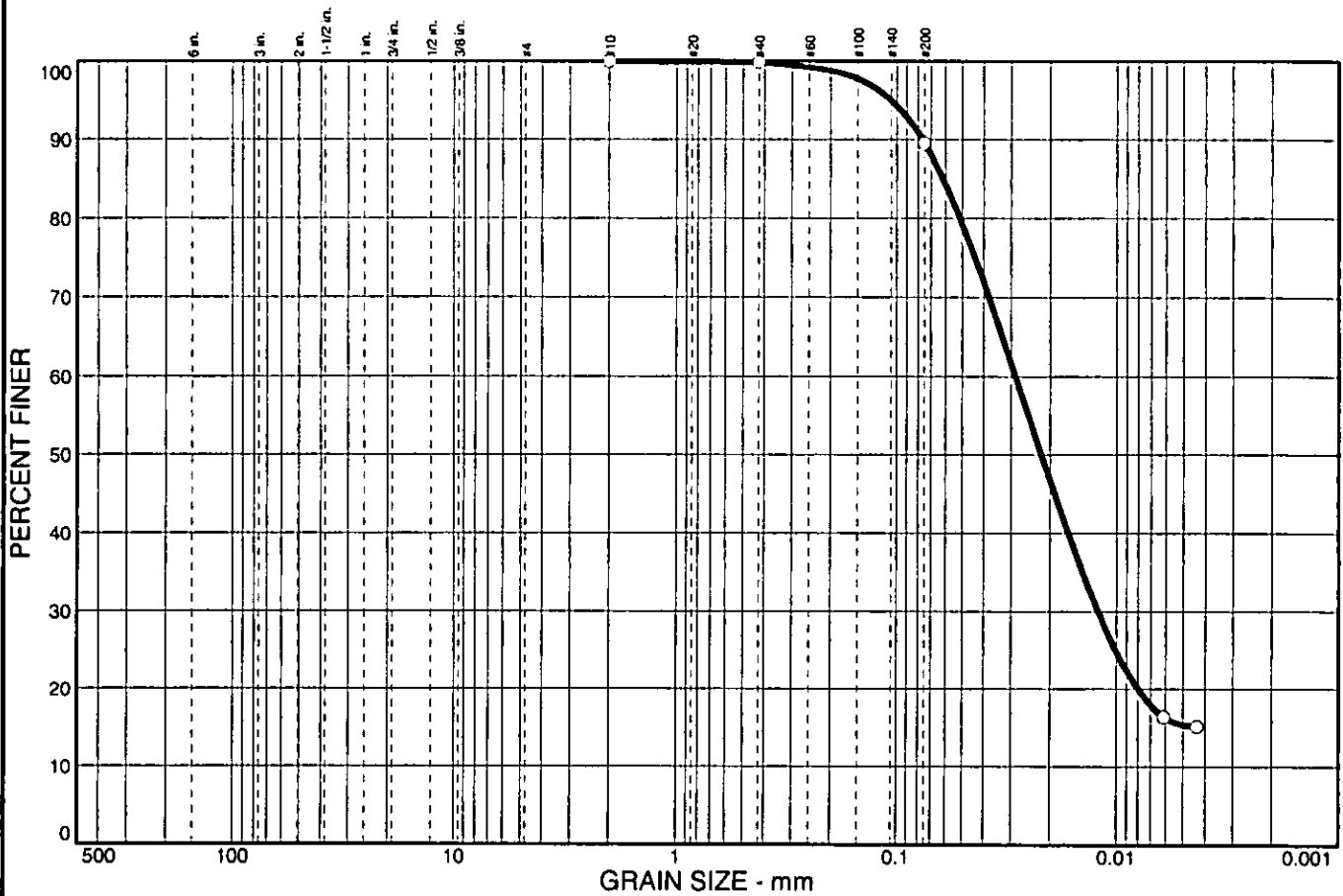
Date: 7/14/06
Elev./Depth: 1.0



Client: TranSystems, Inc.
Project: SCI-823-0.00
Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.1	10.4	74.1	15.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.9		
#200	89.5		

Soil Description

Silty clay

Atterberg Limits

PL= 19 LL= 24 PI= 5

Coefficients

D₈₅= 0.0618 D₆₀= 0.0288 D₅₀= 0.0220
D₃₀= 0.0122 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(3)

Remarks

Moisture Content= 15.1%

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: B-10

Date: 7/21/06
Elev./Depth: 1.0

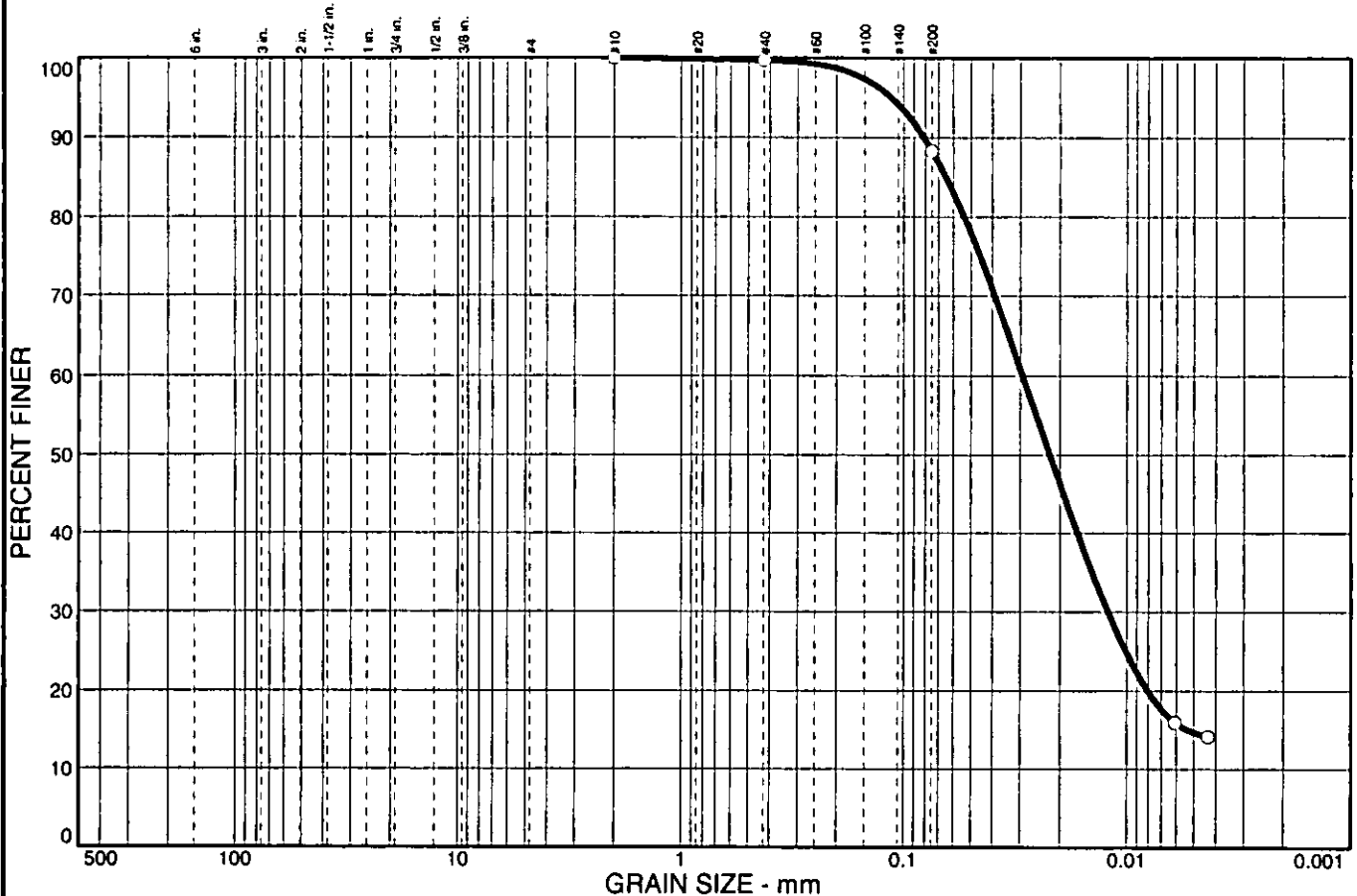


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.3	11.4	73.7	14.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.7		
#200	88.3		

* (no specification provided)

Soil Description

Lean clay

Atterberg Limits

PL= 18 LL= 26 PI= 8

Coefficients

D₈₅= 0.0650 D₆₀= 0.0295 D₅₀= 0.0223
D₃₀= 0.0122 D₁₅= 0.0054 D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(5)

Remarks

Moisture Content= 38.0%

Sample No.: 3
Location:

Source of Sample: B-10

Date: 7/21/06
Elev./Depth: 6.0

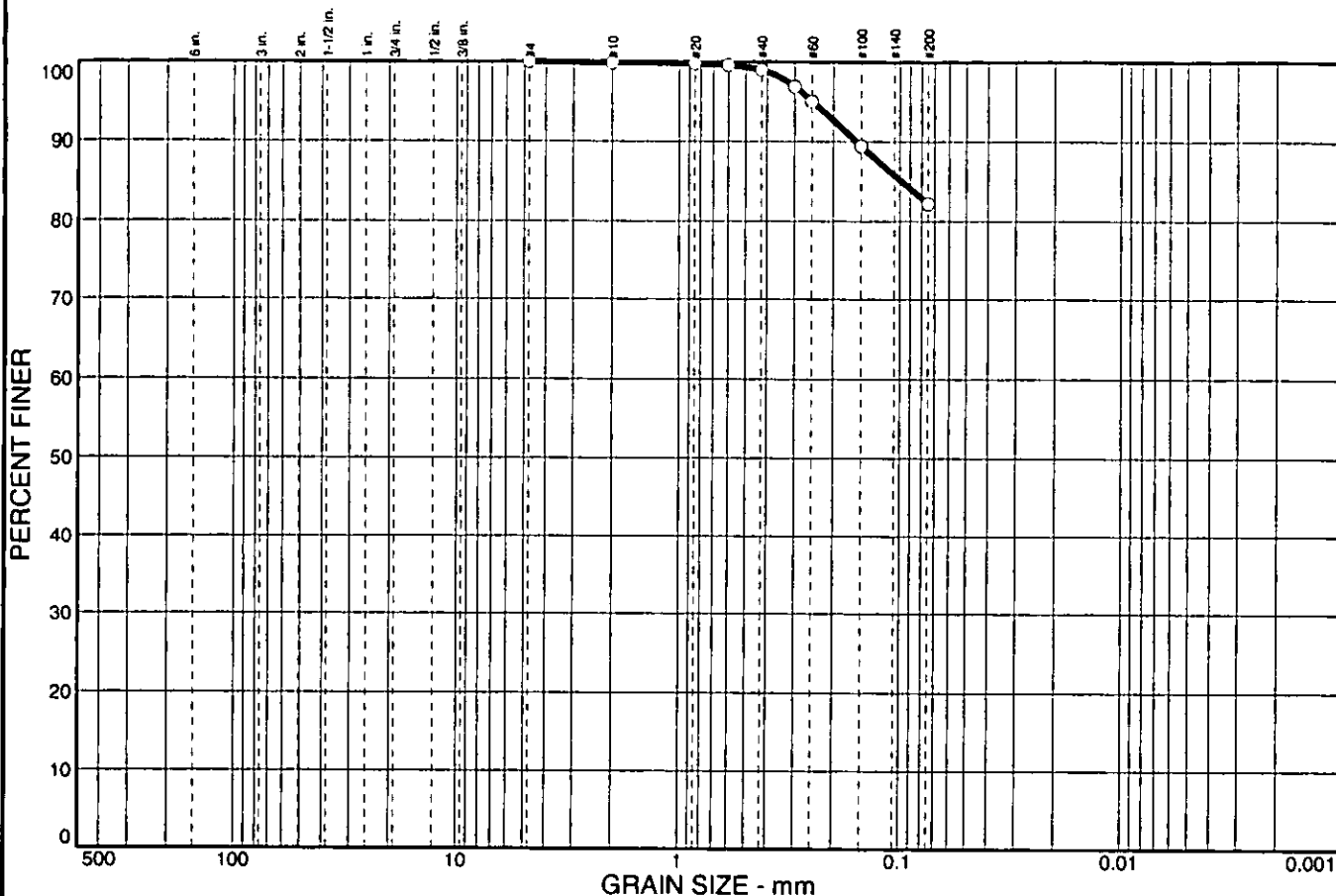


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.1	0.9	16.8	82.2	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.9		
#20	99.8		
#30	99.6		
#40	99.0		
#50	96.9		
#60	95.0		
#100	89.4		
#200	82.2		

Soil Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 0.0986 D₆₀= D₅₀=
 D₃₀= D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content = 13.8%
 F.M.=0.14

* (no specification provided)

Sample No.: 1
 Location:

Source of Sample: B-11

Date: 07/12/06
 Elev./Depth: 1.0

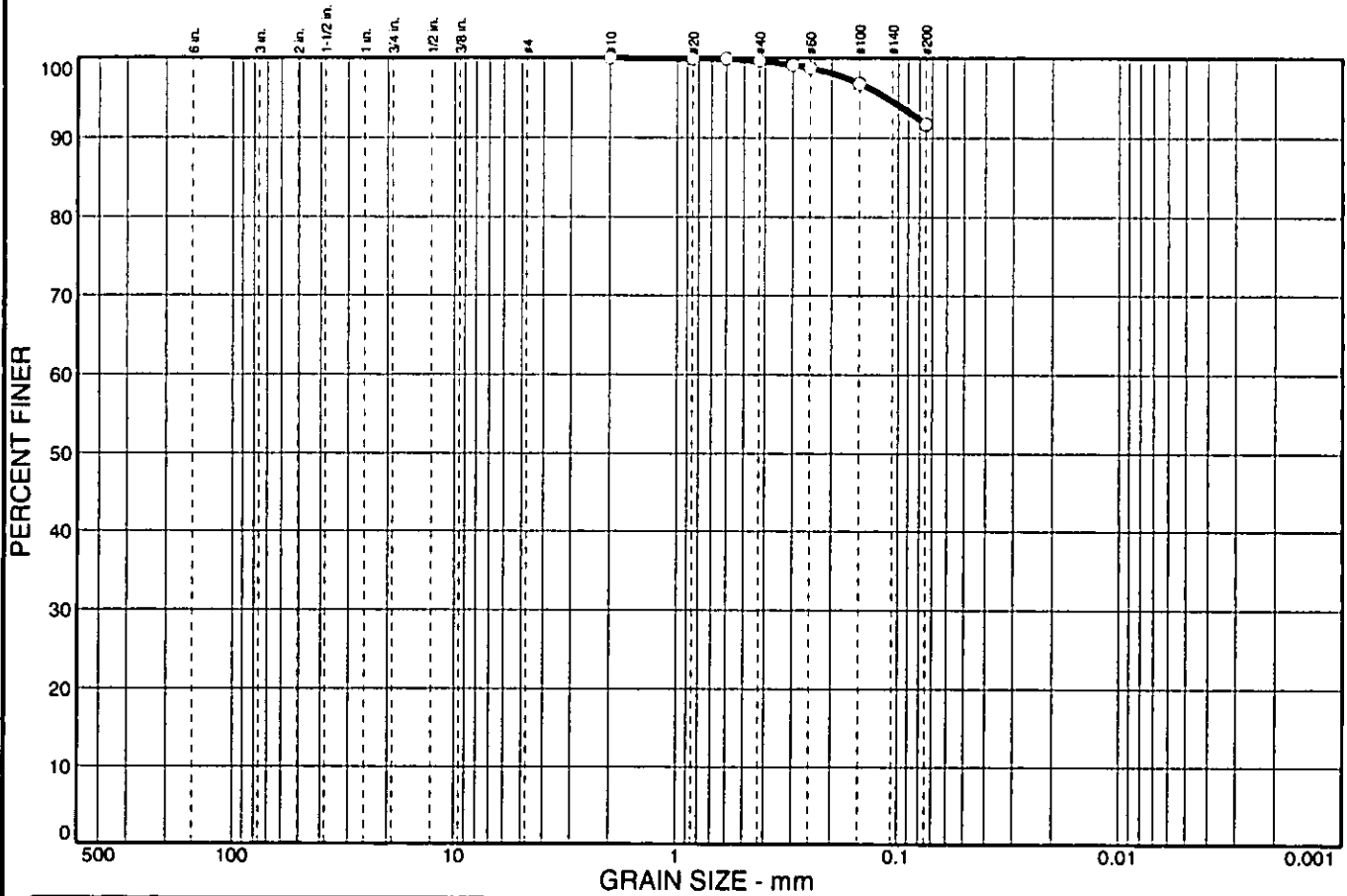


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.4	7.9	91.7	

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#20	99.9		
#30	99.9		
#40	99.6		
#50	99.1		
#60	98.7		
#100	96.7		
#200	91.7		

Soil Description

Silt

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= D₆₀= D₅₀=
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content = 17.0%
F.M.=0.04

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: B-12

Date: 07/12/06
Elev./Depth: 1.0

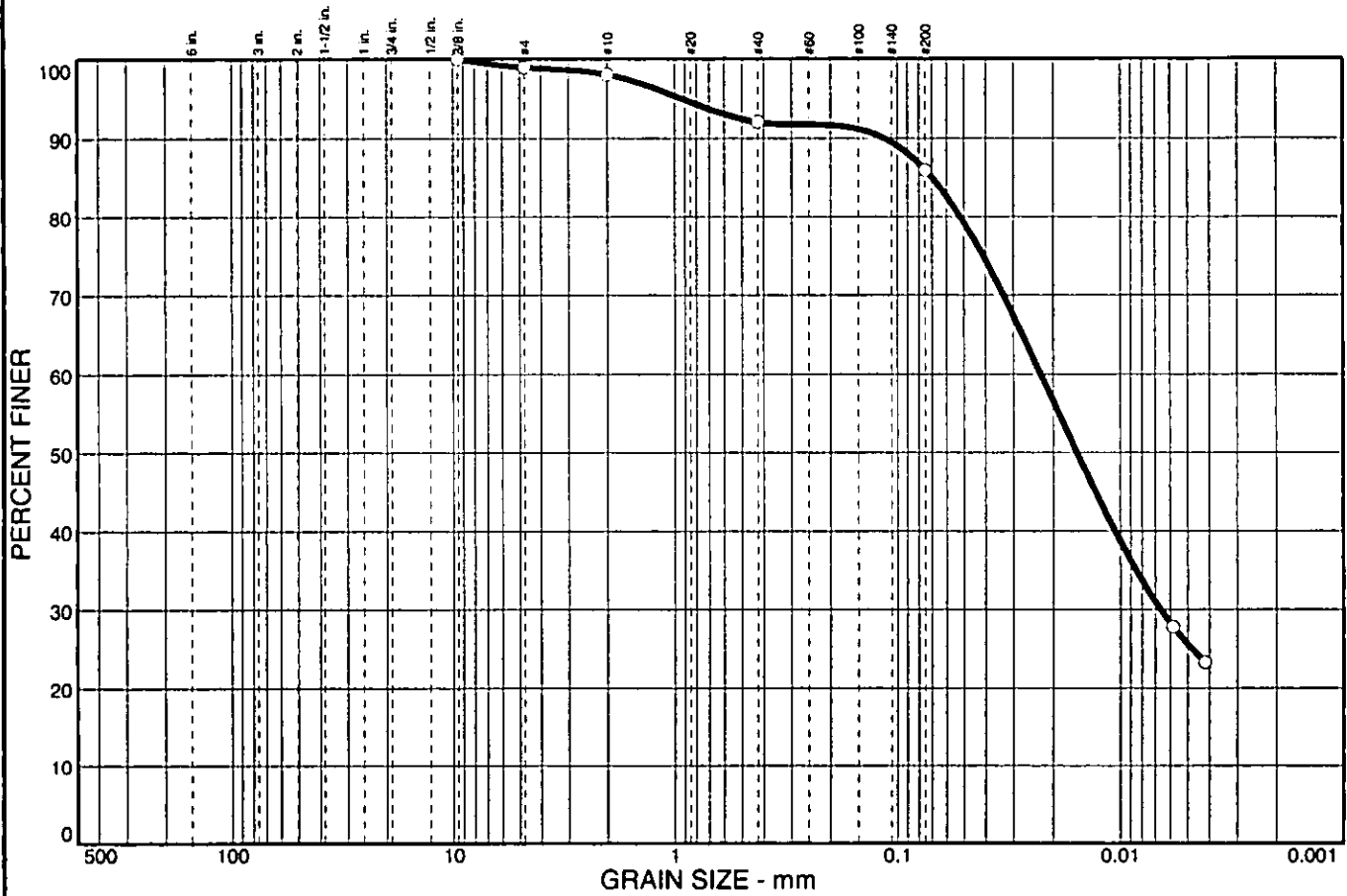


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	1.1	0.9	6.0	6.0	60.4	25.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	98.9		
#10	98.0		
#40	92.0		
#200	86.0		

Soil Description

Lean clay

Atterberg Limits

PL= 17 LL= 26 PI= 9

Coefficients

D₈₅= 0.0698 D₆₀= 0.0228 D₅₀= 0.0157
D₃₀= 0.0066 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(6)

Remarks

Moisture Content= 17.3%
F.M.=0.01

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: C-10

Date: 7/21/06
Elev./Depth: 1.0

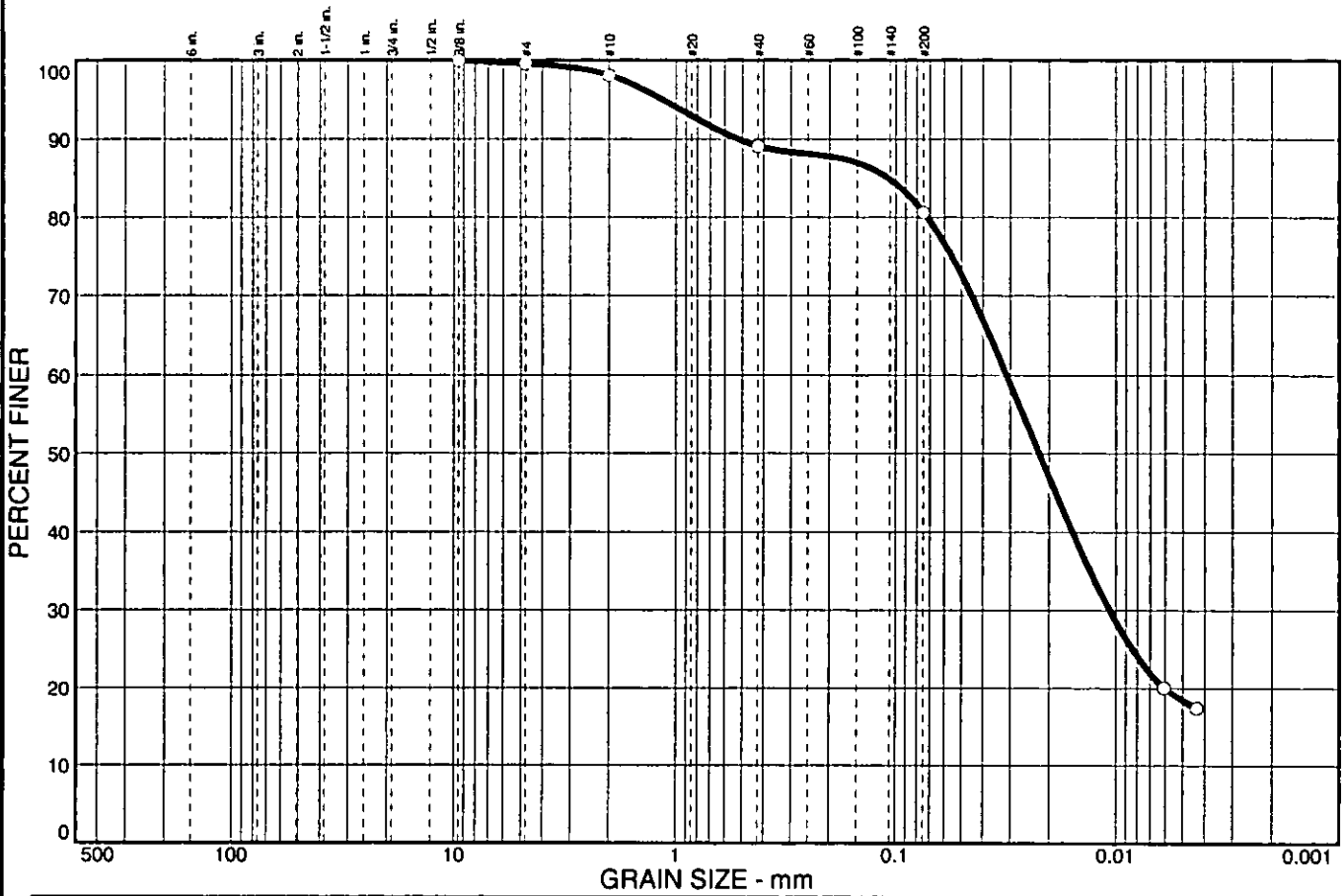


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.4	1.5	9.0	8.5	62.3	18.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.6		
#10	98.1		
#40	89.1		
#200	80.6		

Soil Description

Lean clay with sand

Atterberg Limits

PL= 19 LL= 29 PI= 10

Coefficients

D₈₅= 0.108 D₆₀= 0.0313 D₅₀= 0.0223
D₃₀= 0.0107 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(7)

Remarks

Moisture Content= 15.0%
F.M.=0.00

* (no specification provided)

Sample No.: 2
Location:

Source of Sample: C-10

Date: 7/21/06
Elev./Depth: 3.5

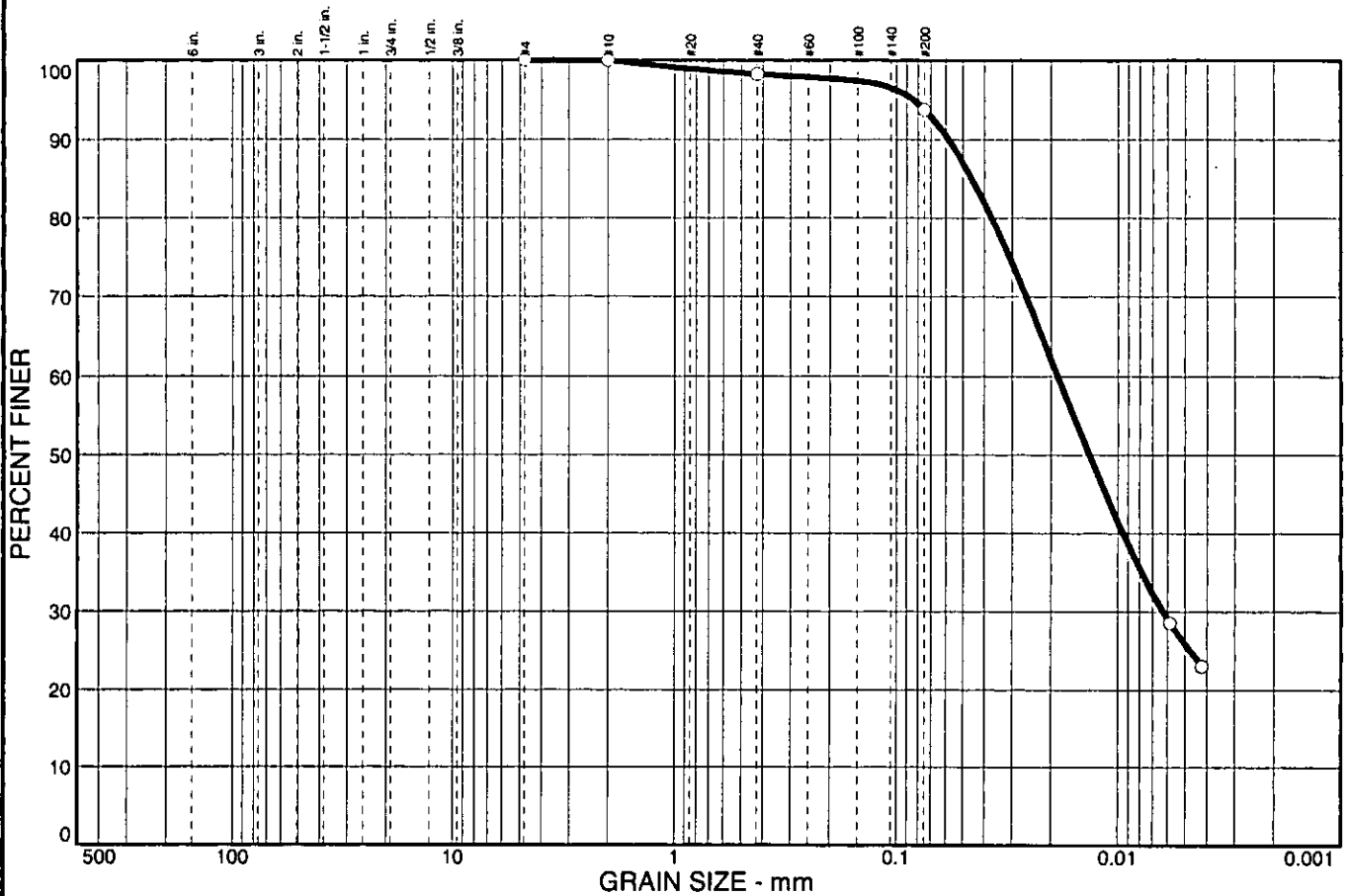


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	1.8	4.5	67.9	25.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	100.0		
#40	98.2		
#200	93.7		

Soil Description

Silty clay

Atterberg Limits

PL= 18 LL= 25 PI= 7

Coefficients

D₈₅= 0.0457 D₆₀= 0.0187 D₅₀= 0.0135
D₃₀= 0.0063 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(5)

Remarks

Moisture Content= 18.5%

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: C-11

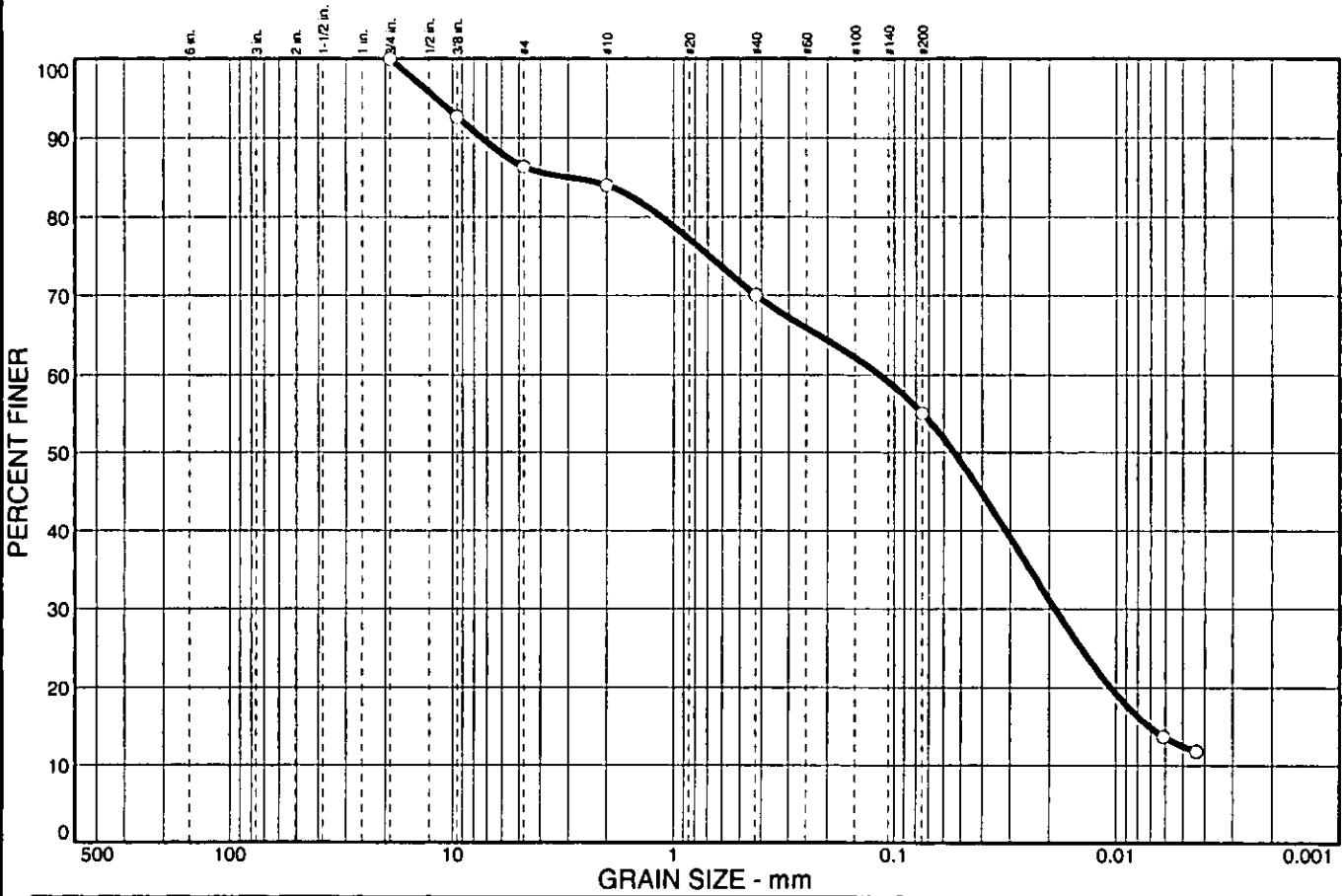
Date: 7/21/06
Elev./Depth: 1.0



Client: TranSystems, Inc.
Project: SCI-823-0.00
Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	13.6	2.4	14.0	15.0	42.5	12.5

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	92.6		
#4	86.4		
#10	84.0		
#40	70.0		
#200	55.0		

Soil Description

Sandy silty clay

Atterberg Limits

PL= 21 LL= 27 PI= 6

Coefficients

D₈₅= 2.95 D₆₀= 0.117 D₅₀= 0.0541
 D₃₀= 0.0189 D₁₅= 0.0071 D₁₀=
 C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(1)

Remarks

Moisture Content= 14.4%
 F.M.=0.21

* (no specification provided)

Sample No.: 3
 Location:

Source of Sample: C-11

Date: 7/21/06
 Elev./Depth: 6.0

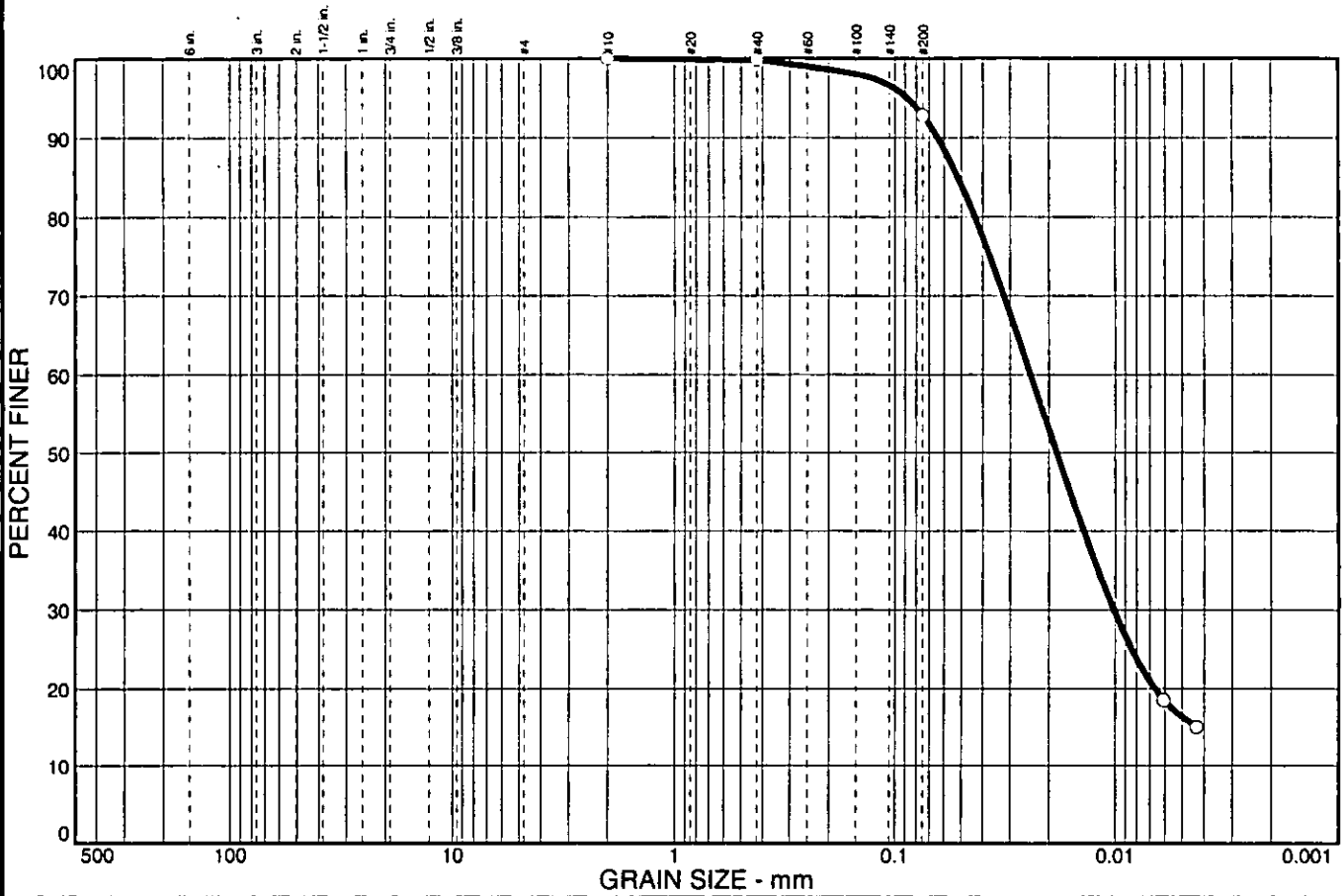


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.2	7.0	76.5	16.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.8		
#200	92.8		

(no specification provided)

Soil Description

Silt

Atterberg Limits

PL= 24 LL= 30 PI= 6

Coefficients

D₈₅= 0.0522 D₆₀= 0.0244 D₅₀= 0.0185
D₃₀= 0.0102 D₁₅= 0.0043 D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(6)

Remarks

Moisture Content= 15.1%

Sample No.: 5
Location:

Source of Sample: C-11

Date: 7/21/06
Elev./Depth: 11.0

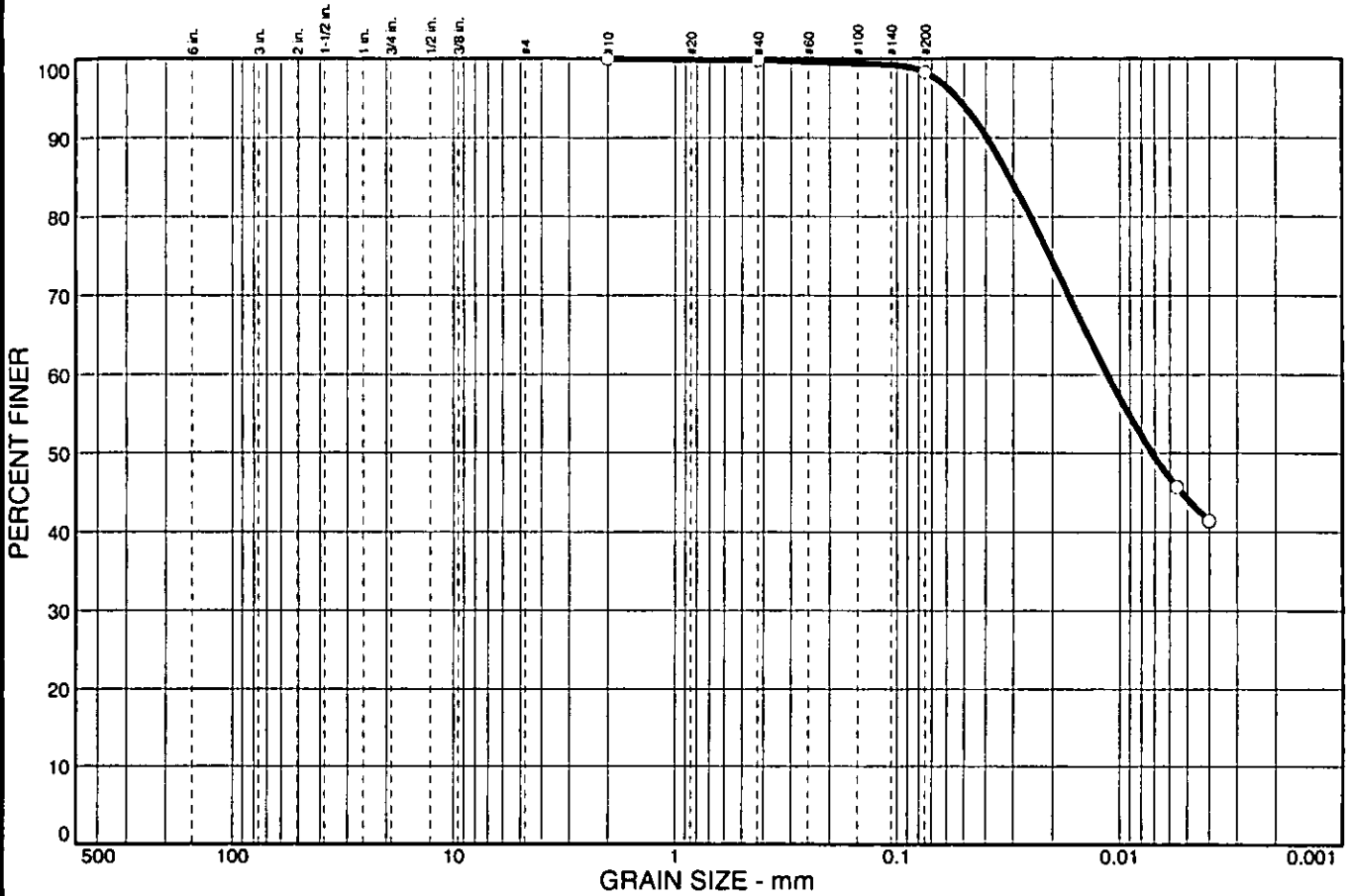


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.2	1.5	54.2	44.1

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.8		
#200	98.3		

Soil Description

Lean clay

Atterberg Limits

PL= 19 LL= 41 PI= 22

Coefficients

D₈₅= 0.0313 D₆₀= 0.0114 D₅₀= 0.0072
 D₃₀= D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= CL AASHTO= A-7-6(23)

Remarks

Moisture Content= 23.3%

(no specification provided)

Sample No.: 2
Location:

Source of Sample: C-12

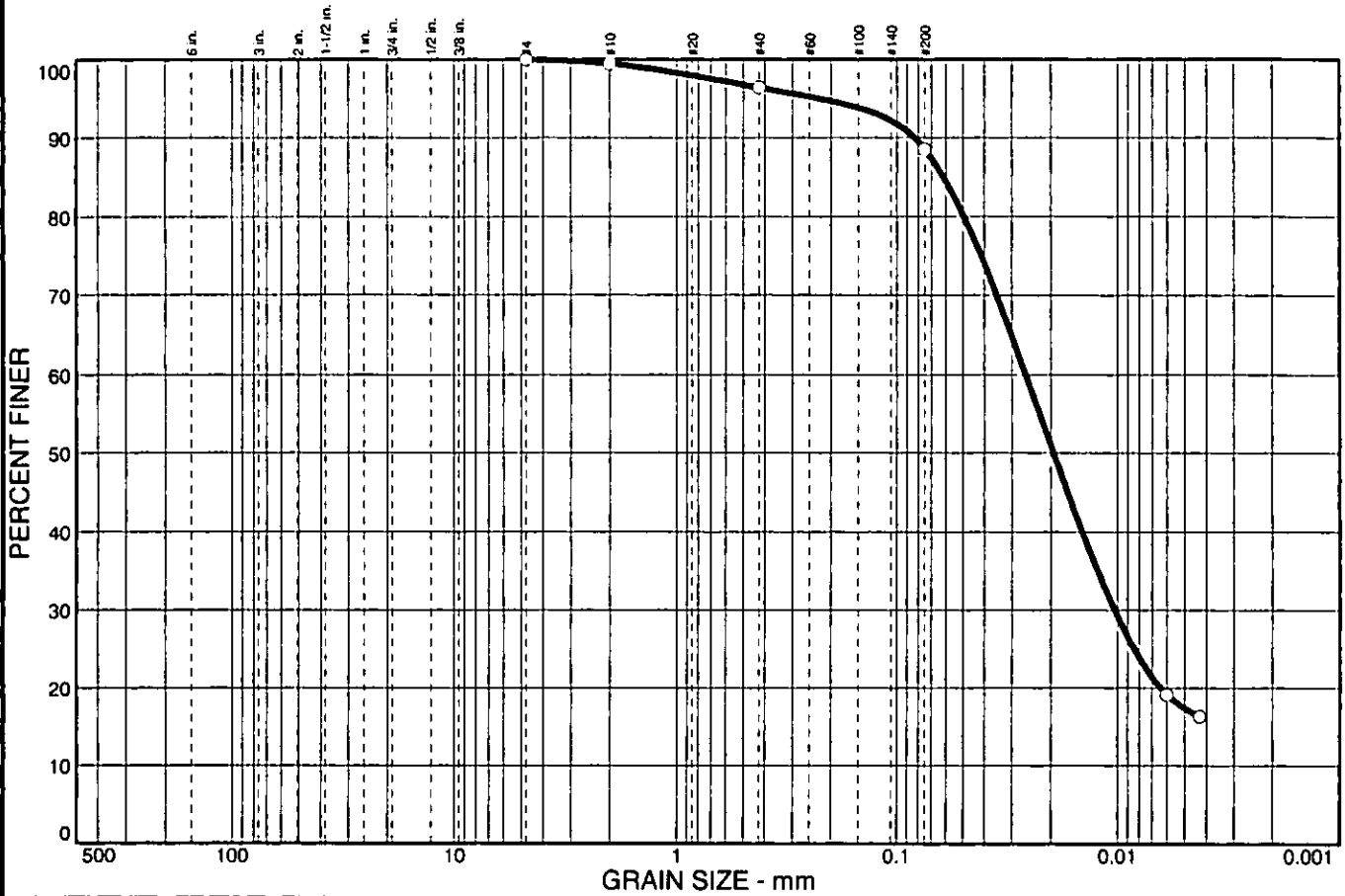
Date: 7/21/06
Elev./Depth: 3.5



Client: TranSystems, Inc.
Project: SCI-823-0.00
Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.5	3.1	7.9	71.2	17.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.5		
#40	96.4		
#200	88.5		

(no specification provided)

Soil Description

Silty clay

Atterberg Limits

PL= 17 LL= 21 PI= 4

Coefficients

D₈₅= 0.0617 D₆₀= 0.0261 D₅₀= 0.0195
 D₃₀= 0.0103 D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(1)

Remarks

Moisture Content= 16.6%

Sample No.: 4
Location:

Source of Sample: C-12

Date: 7/21/06
Elev./Depth: 8.5

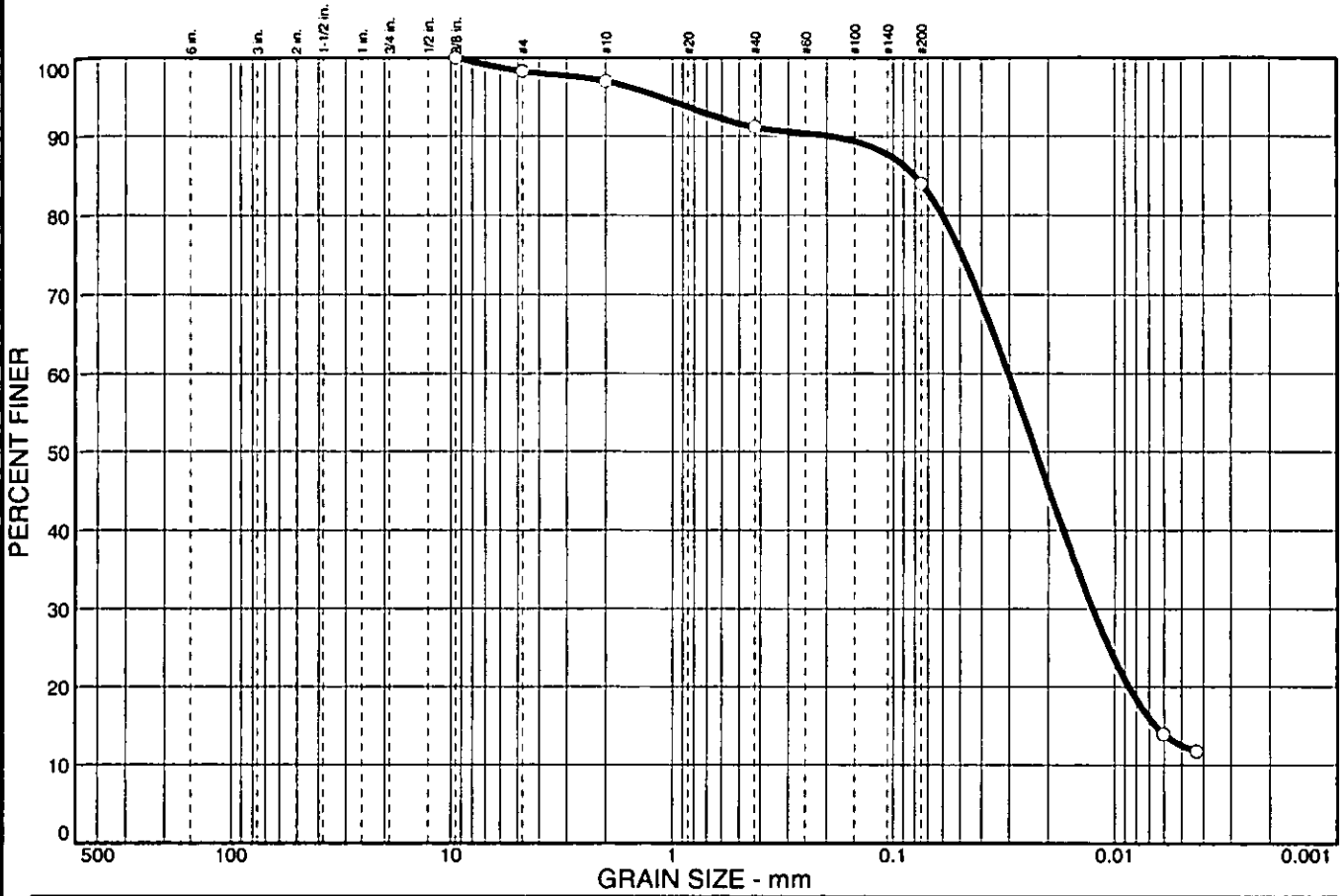


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	1.7	1.3	5.9	7.1	71.6	12.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	98.3		
#10	97.0		
#40	91.1		
#200	84.0		

Soil Description

Silt with sand

Atterberg Limits

PL= 26 LL= 30 PI= 4

Coefficients

D₈₅= 0.0805 D₆₀= 0.0305 D₅₀= 0.0229
D₃₀= 0.0126 D₁₅= 0.0066 D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(3)

Remarks

Moisture Content = 19.5%
F.M.=0.02

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: C-13

Date: 07/19/06
Elev./Depth: 1.0

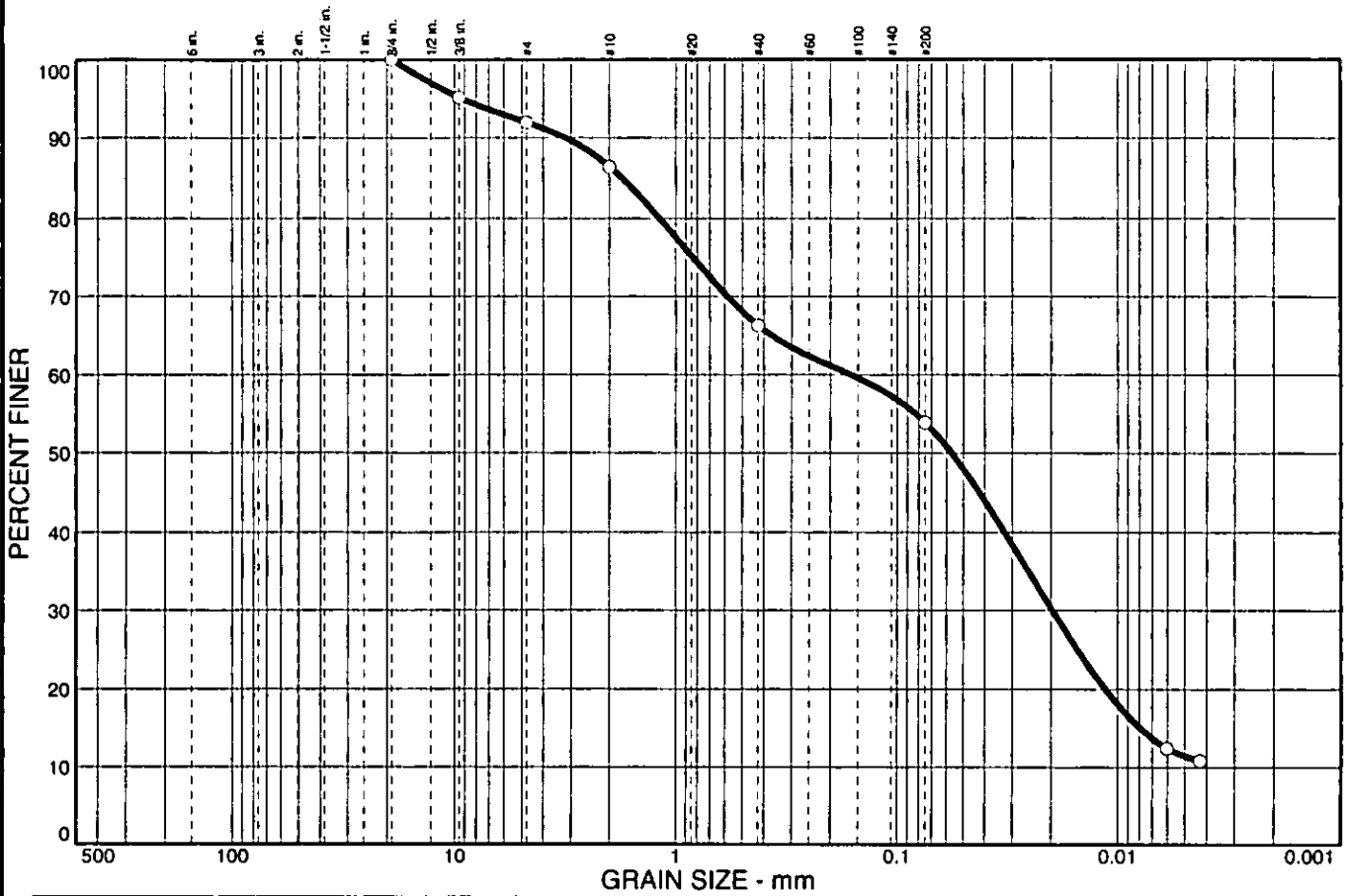


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	8.1	5.5	20.1	12.5	42.4	11.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	95.1		
#4	91.9		
#10	86.4		
#40	66.3		
#200	53.8		

Soil Description

Sandy silt

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 1.75 D₆₀= 0.162 D₅₀= 0.0571
 D₃₀= 0.0198 D₁₅= 0.0080 D₁₀=
 C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content = 12.5%
 F.M.=0.13

(no specification provided)

Sample No.: 2
 Location:

Source of Sample: C-13

Date: 07/19/06
 Elev./Depth: 3.5

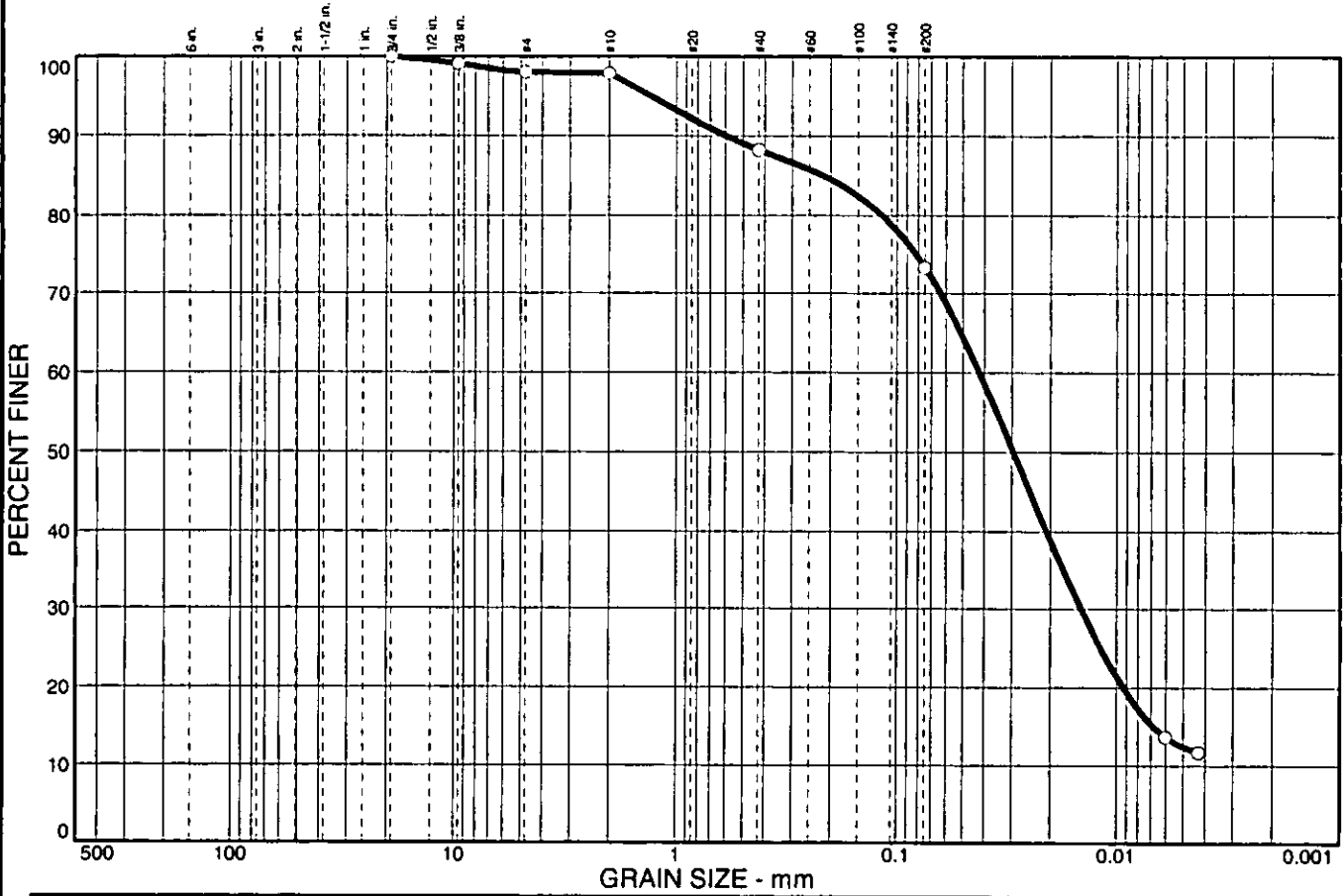


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	2.0	0.2	9.6	14.9	60.9	12.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	99.1		
#4	98.0		
#10	97.8		
#40	88.2		
#200	73.3		

Soil Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 0.215 D₆₀= 0.0424 D₅₀= 0.0297
 D₃₀= 0.0145 D₁₅= 0.0068 D₁₀=
 C_u=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content = 15.4%
 F.M.=0.03

* (no specification provided)

Sample No.: 5
 Location:

Source of Sample: C-13

Date: 07/19/06
 Elev./Depth: 11.0

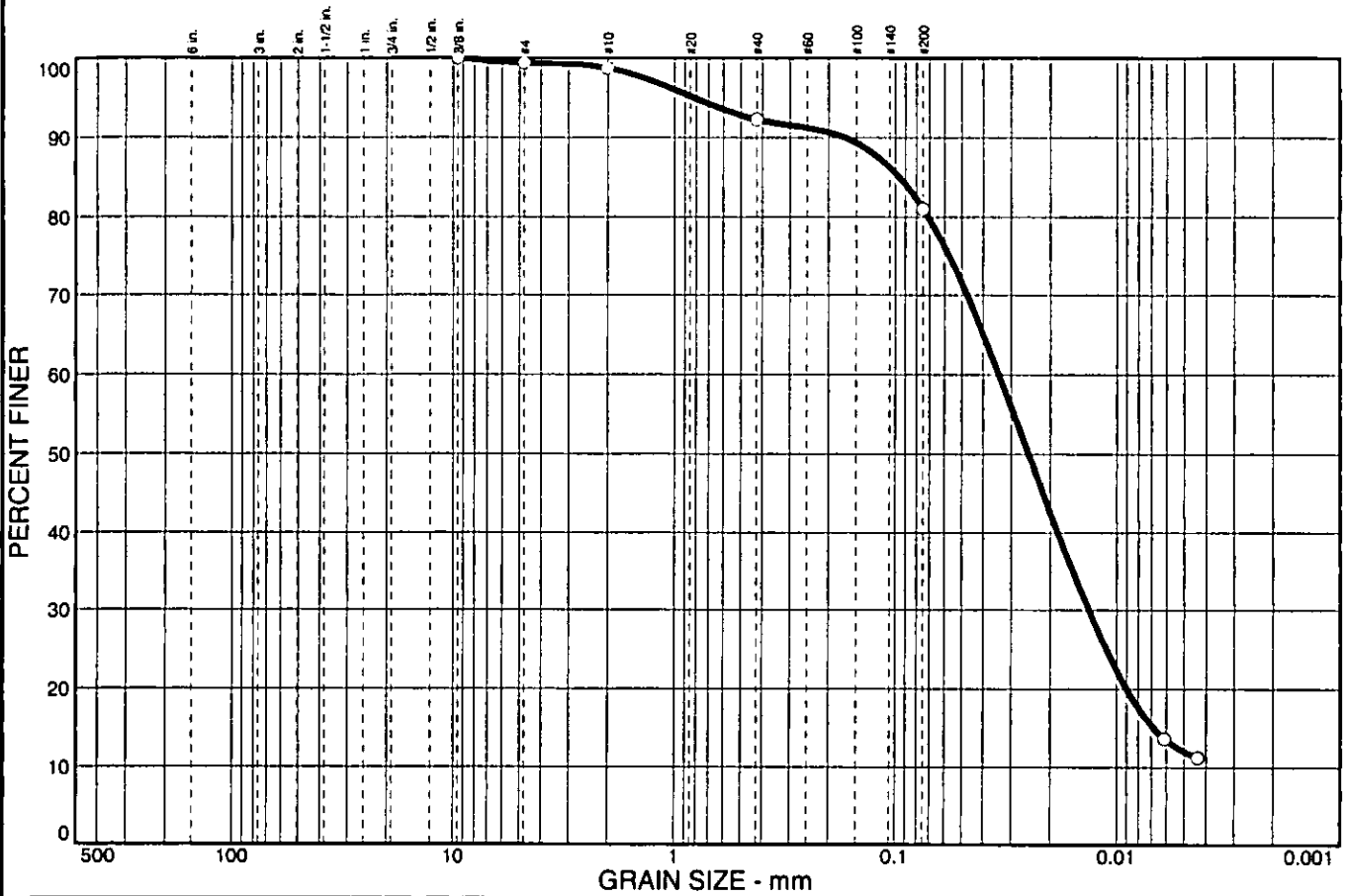


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.6	0.7	6.5	11.2	69.0	12.0

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.4		
#10	98.7		
#40	92.2		
#200	81.0		

(no specification provided)

Soil Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 0.0958 D₆₀= 0.0342 D₅₀= 0.0252
D₃₀= 0.0134 D₁₅= 0.0069 D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 16.9%
F.M.=0.01

Sample No.: 6
Location:

Source of Sample: C-13

Date: 7/19/06
Elev./Depth: 13.5

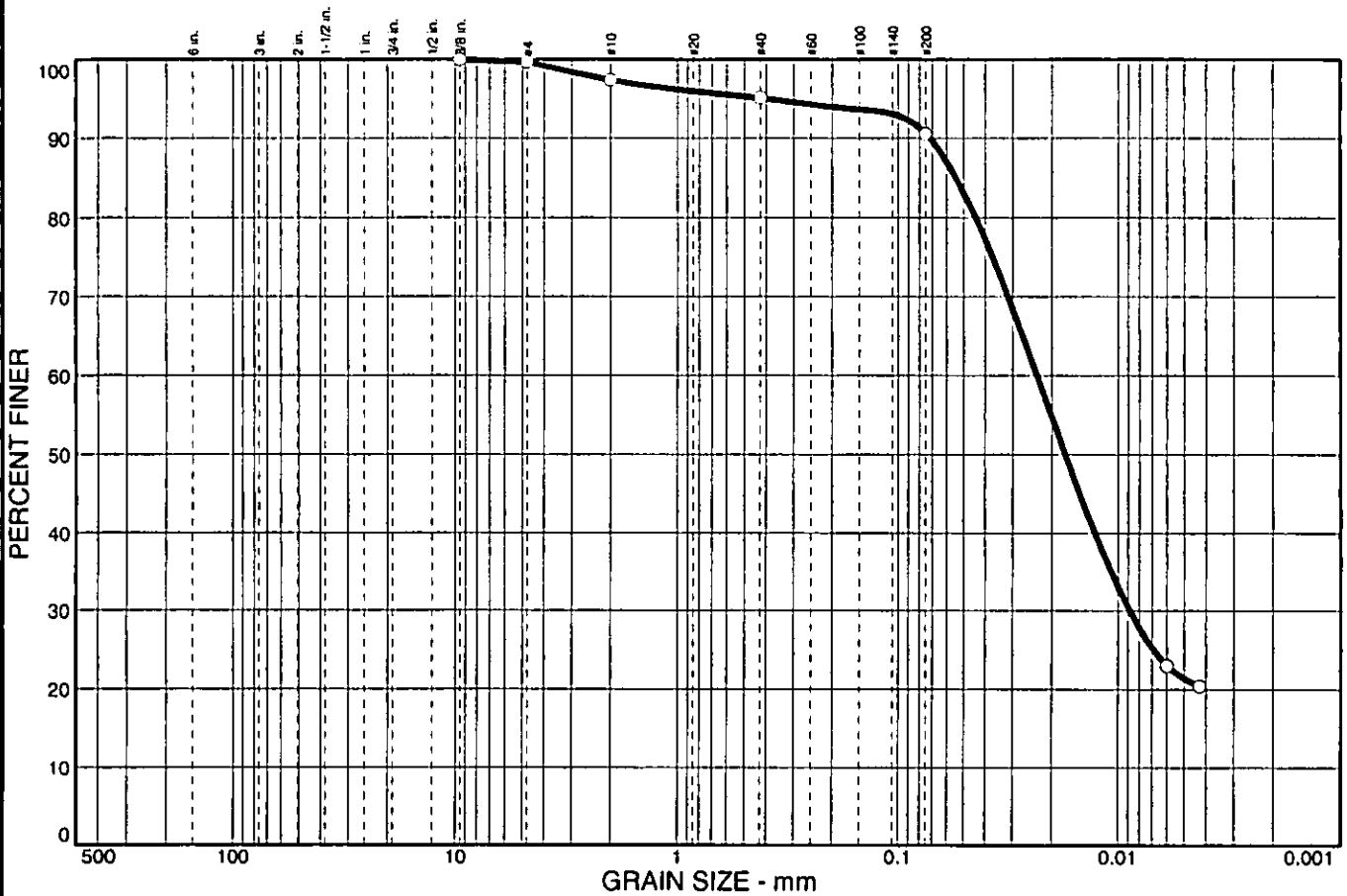


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.3	2.4	2.3	4.5	69.1	21.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.7		
#10	97.3		
#40	95.0		
#200	90.5		

(no specification provided)

Soil Description

Silty clay

Atterberg Limits

PL= 20 LL= 27 PI= 7

Coefficients

D₈₅= 0.0545 D₆₀= 0.0234 D₅₀= 0.0175
D₃₀= 0.0089 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(5)

Remarks

Moisture Content= 15.8%
F.M.=0.00

Sample No.: I
Location:

Source of Sample: C-14

Date: 7/19/06
Elev./Depth: 1.0

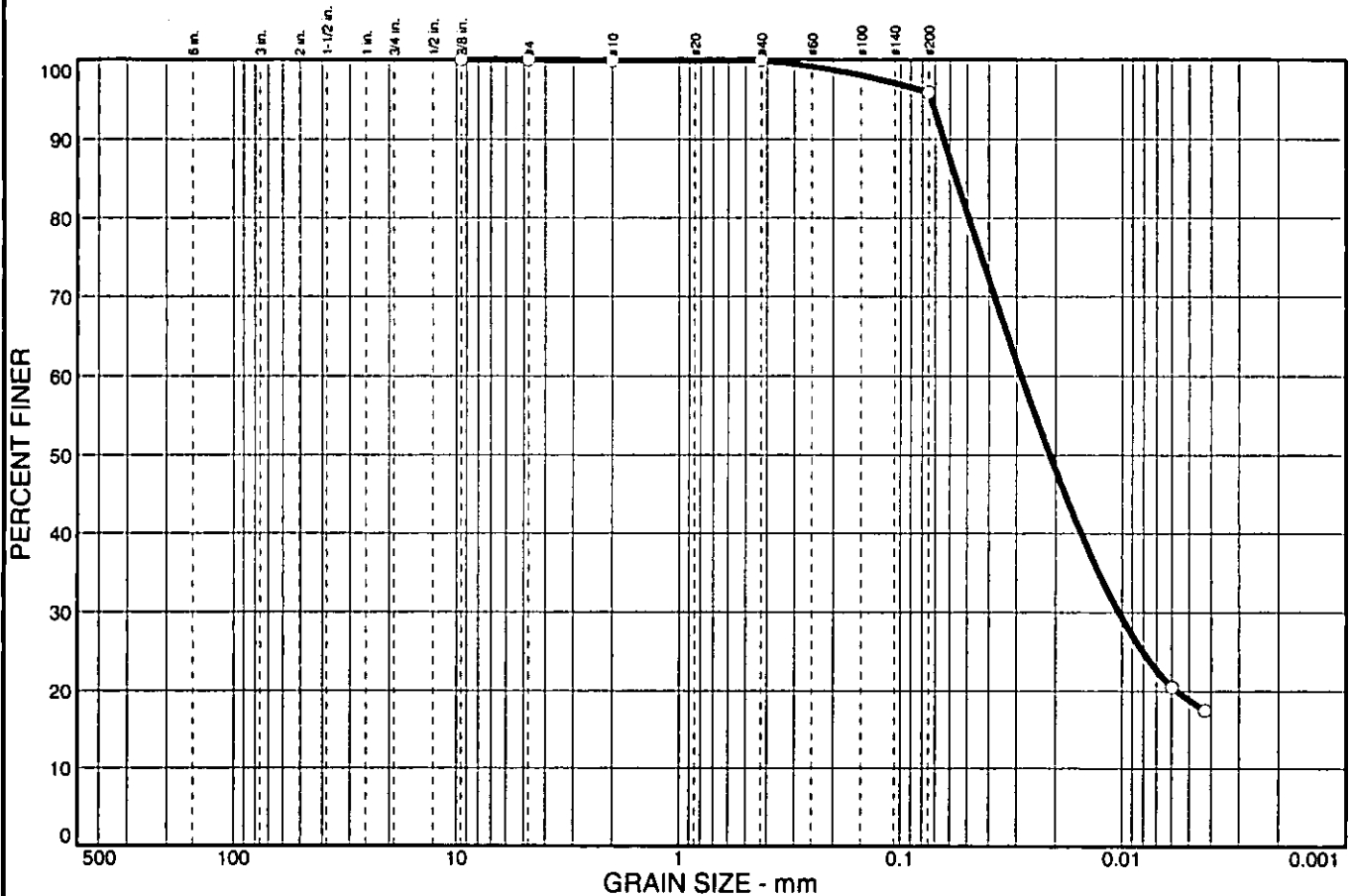


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.1	0.0	4.1	77.0	18.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	100.0		
#10	99.9		
#40	99.9		
#200	95.8		

Soil Description

Silty clay

Atterberg Limits

PL= 18 LL= 24 PI= 6

Coefficients

D₈₅= 0.0565 D₆₀= 0.0287 D₅₀= 0.0213
D₃₀= 0.0104 C_u= D₁₀=

Classification

USCS= CL-ML AASHTO= A-4(4)

Remarks

Moisture Content = 18.2%

* (no specification provided)

Sample No.: 3
Location:

Source of Sample: C-14

Date: 07/19/06
Elev./Depth: 6.0

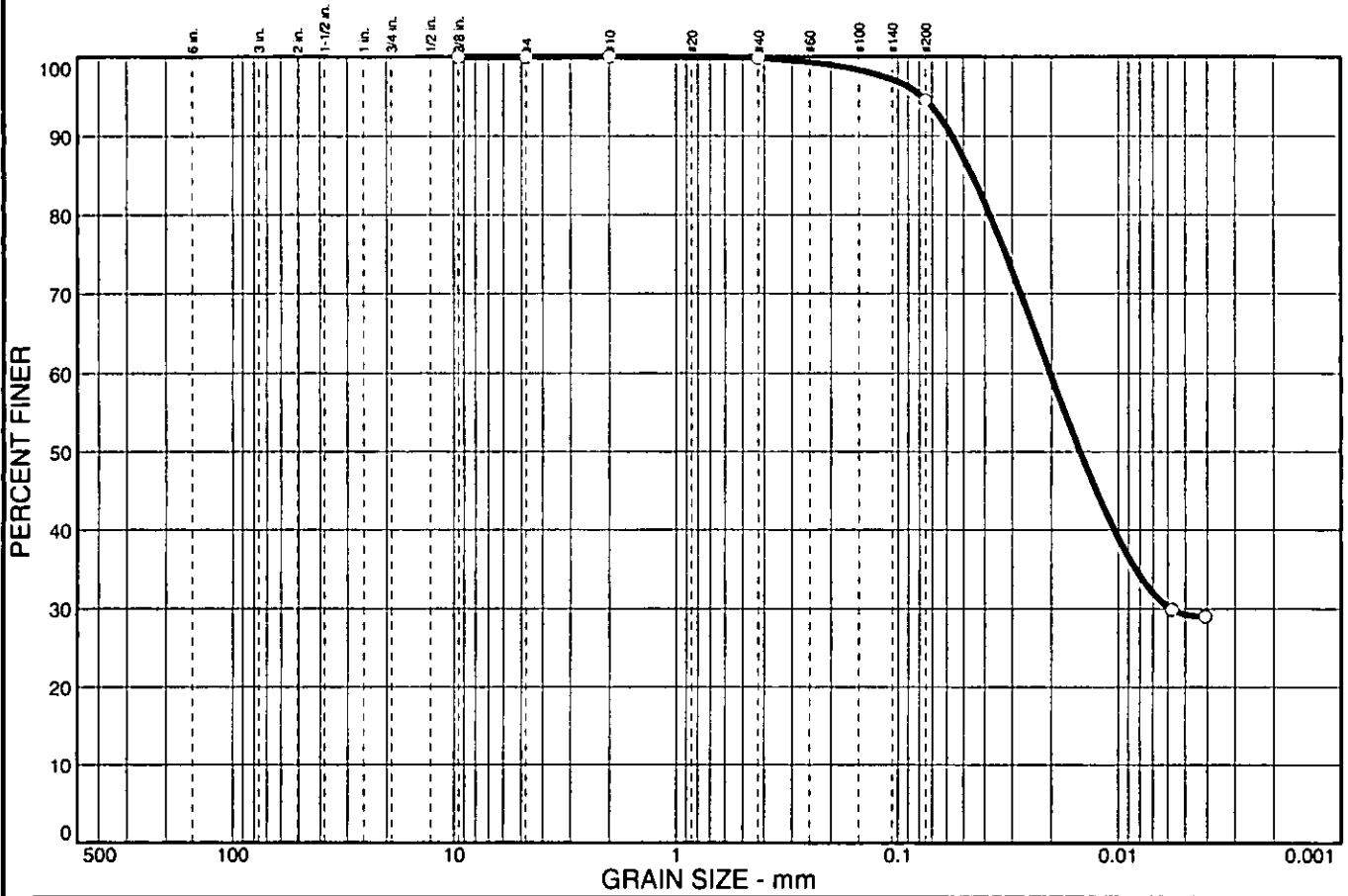


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.1	5.4	65.3	29.2

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	100.0		
#10	100.0		
#40	99.9		
#200	94.5		

Soil Description

Lean clay

Atterberg Limits

PL= 18 LL= 27 PI= 9

Coefficients

D₈₅= 0.0459 D₆₀= 0.0204 D₅₀= 0.0149
D₃₀= 0.0058 C_u= D₁₀=

Classification

USCS= CL AASHTO= A-4(7)

Remarks

Moisture Content = 19.1%

(no specification provided)

Sample No.: 4
Location:

Source of Sample: C-14

Date: 07/19/06
Elev./Depth: 8.5

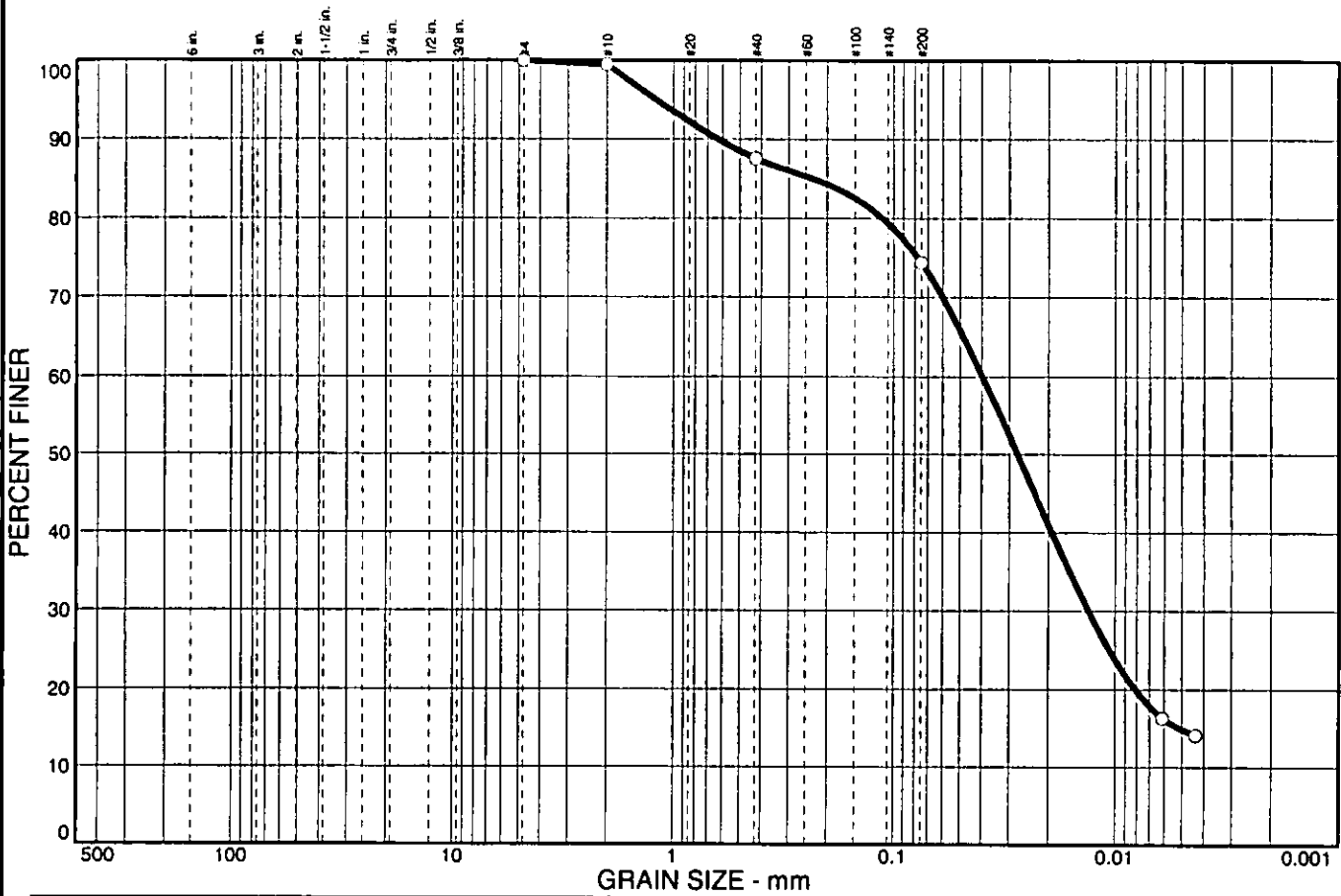


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.5	11.9	13.3	59.5	14.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.5		
#40	87.6		
#200	74.3		

(no specification provided)

Soil Description

Lean clay with sand

Atterberg Limits

PL= 20 LL= 30 PI= 10

Coefficients

D₈₅= 0.231 D₆₀= 0.0399 D₅₀= 0.0277
D₃₀= 0.0132 D₁₅= 0.0051 D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(6)

Remarks

Moisture Content= 18.4%

Sample No.: 1
Location:

Source of Sample: C-15

Date: 7/21/06
Elev./Depth: 1.0

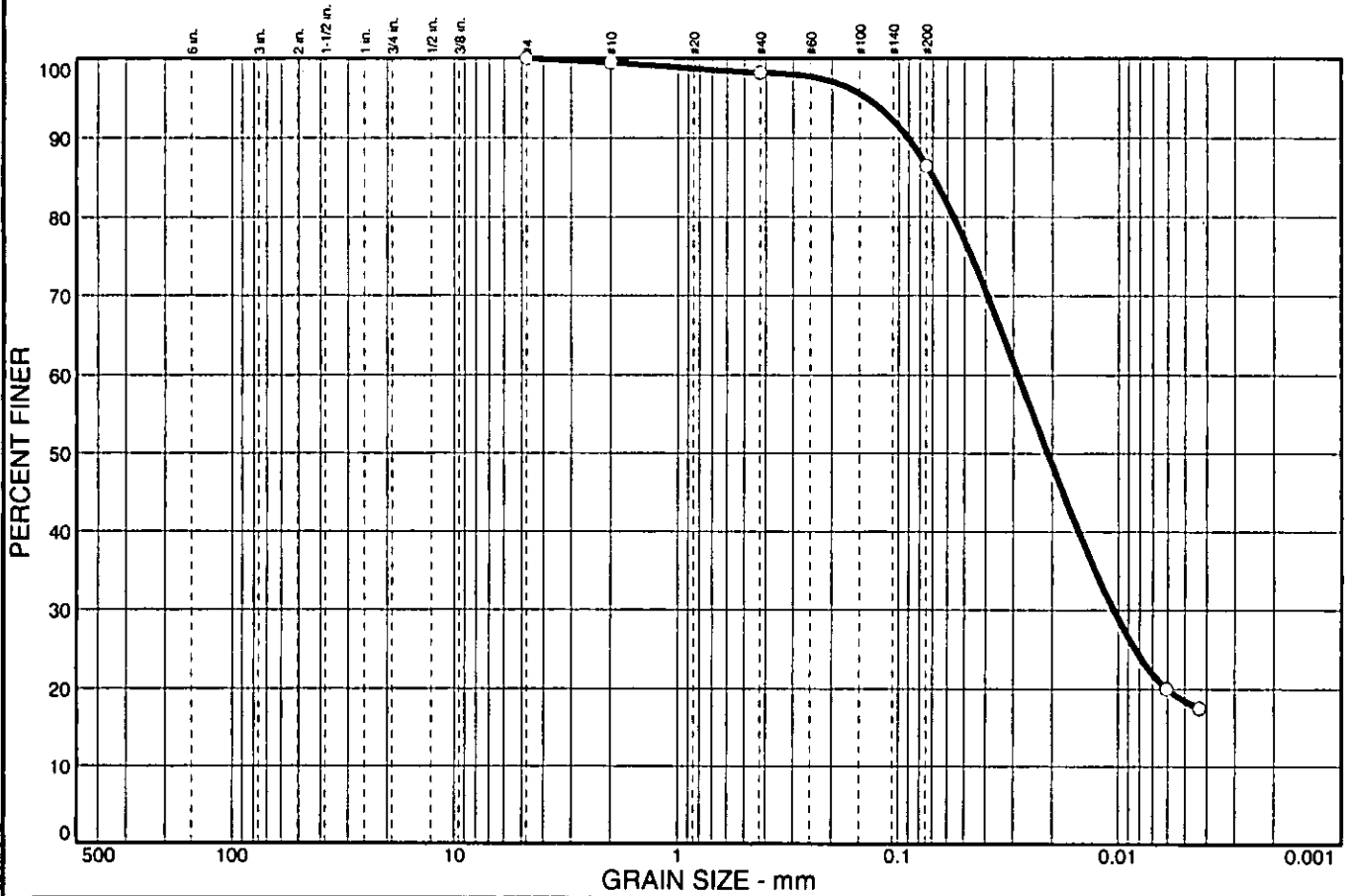


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.5	1.3	11.7	68.1	18.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.5		
#40	98.2		
#200	86.5		

Soil Description

Lean clay

Atterberg Limits

PL= 21 LL= 30 PI= 9

Coefficients

D₈₅= 0.0697 D₆₀= 0.0287 D₅₀= 0.0210
D₃₀= 0.0105 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(7)

Remarks

Moisture Content= 13.6%

* (no specification provided)

Sample No.: 2
Location:

Source of Sample: C-15

Date: 7/21/06
Elev./Depth: 3.5

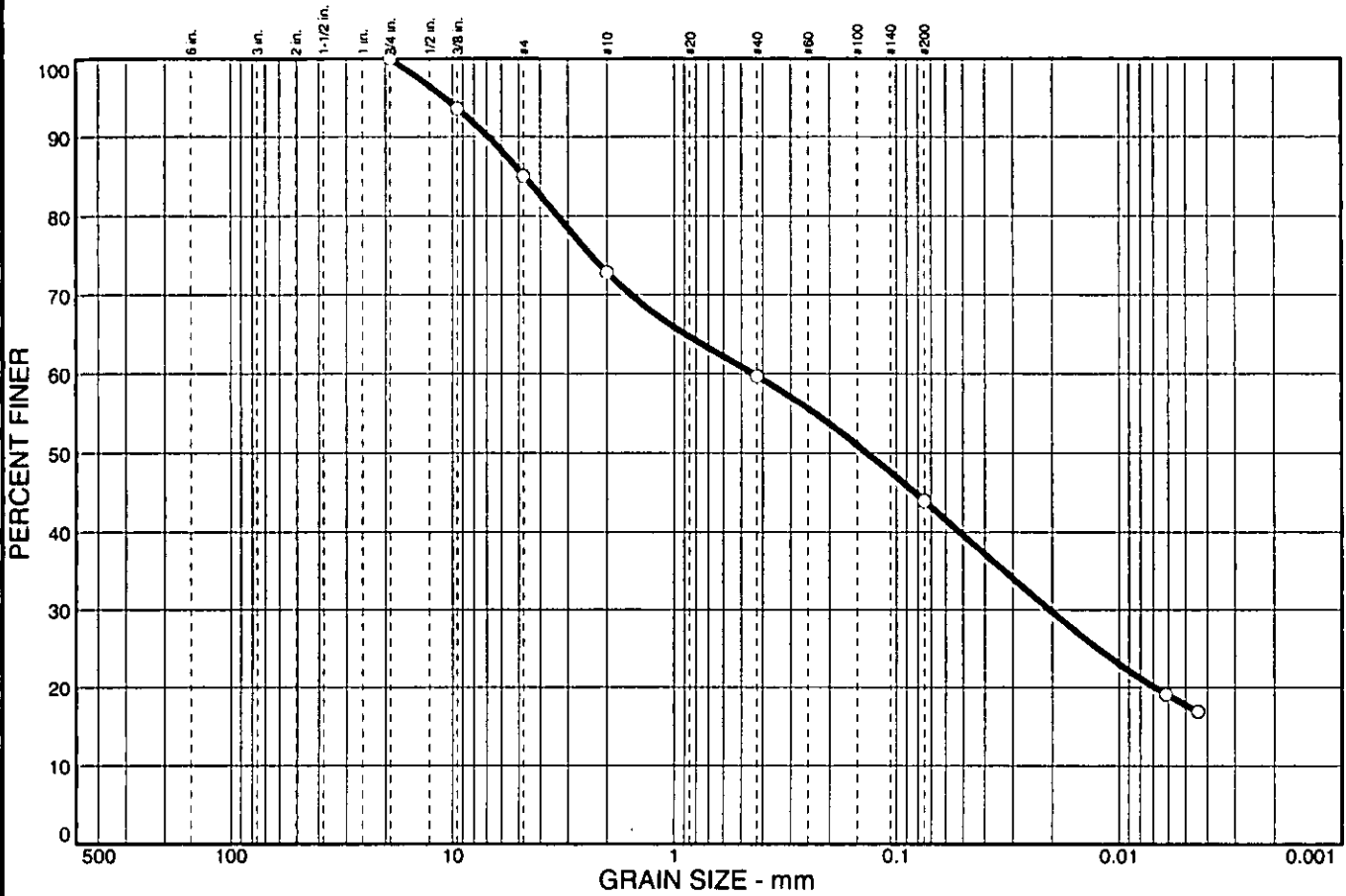


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	14.9	12.3	13.1	15.8	26.2	17.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	93.5		
#4	85.1		
#10	72.8		
#40	59.7		
#200	43.9		

Soil Description

Clayey sand

Atterberg Limits

PL= 15 LL= 23 PI= 8

Coefficients

D₈₅= 4.72 D₆₀= 0.443 D₅₀= 0.137
D₃₀= 0.0206 C_u= D₁₀=

Classification

USCS= SC AASHTO= A-4(0)

Remarks

Moisture Content= 10.9%
F.M.=0.21

* (no specification provided)

Sample No.: 2
Location:

Source of Sample: R-360

Date: 2/16/05
Elev./Depth: 3.5

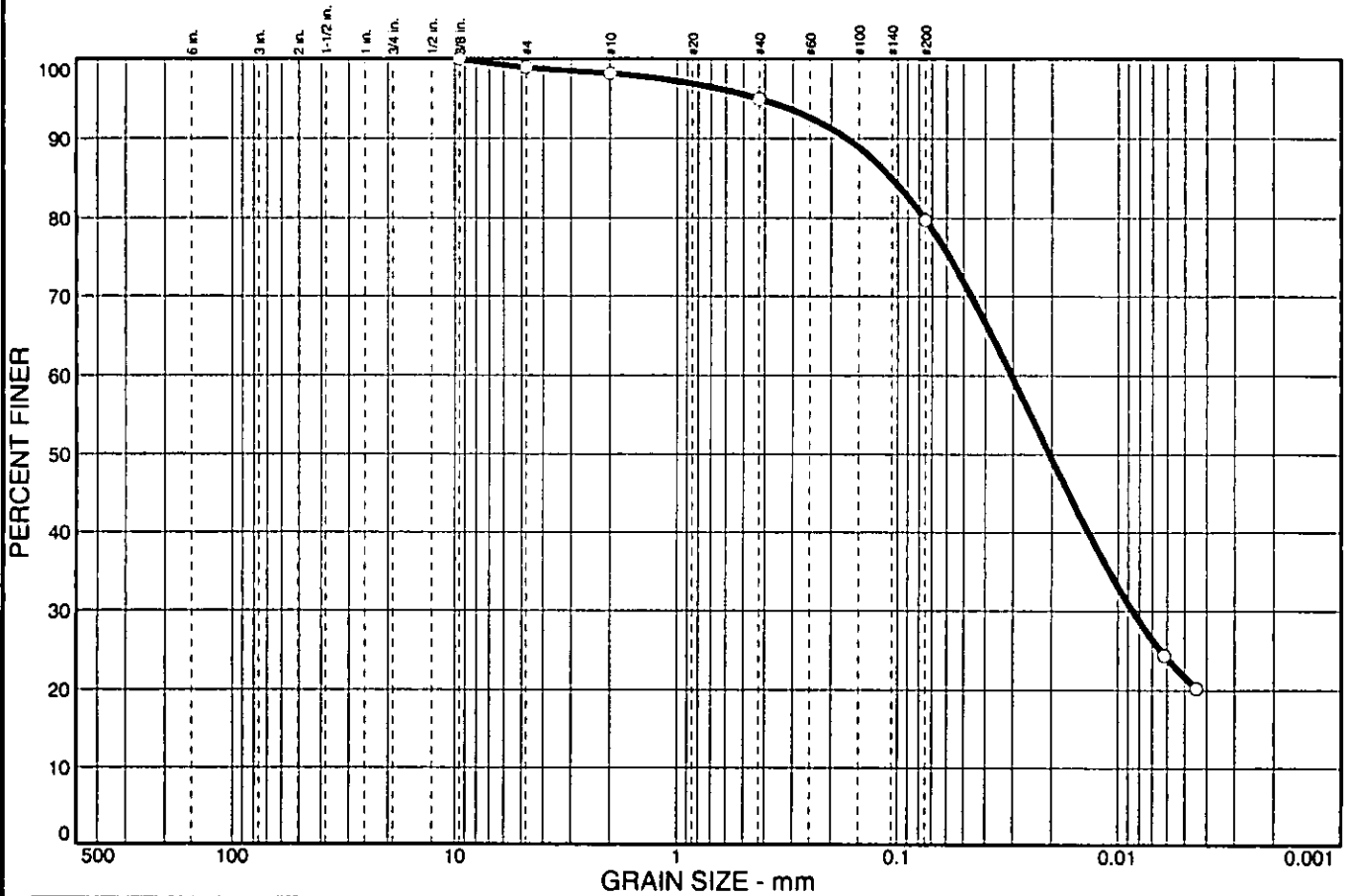


Client: TranSystems, Inc.
Project: SC1-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	1.1	0.8	3.2	15.2	58.1	21.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	98.9		
#10	98.1		
#40	94.9		
#200	79.7		

Soil Description

Silty clay with sand

Atterberg Limits

PL= 16 LL= 21 PI= 5

Coefficients

D₈₅= 0.107 D₆₀= 0.0306 D₅₀= 0.0206
D₃₀= 0.0086 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(1)

Remarks

Moisture Content= 9.9%
F.M.=0.01

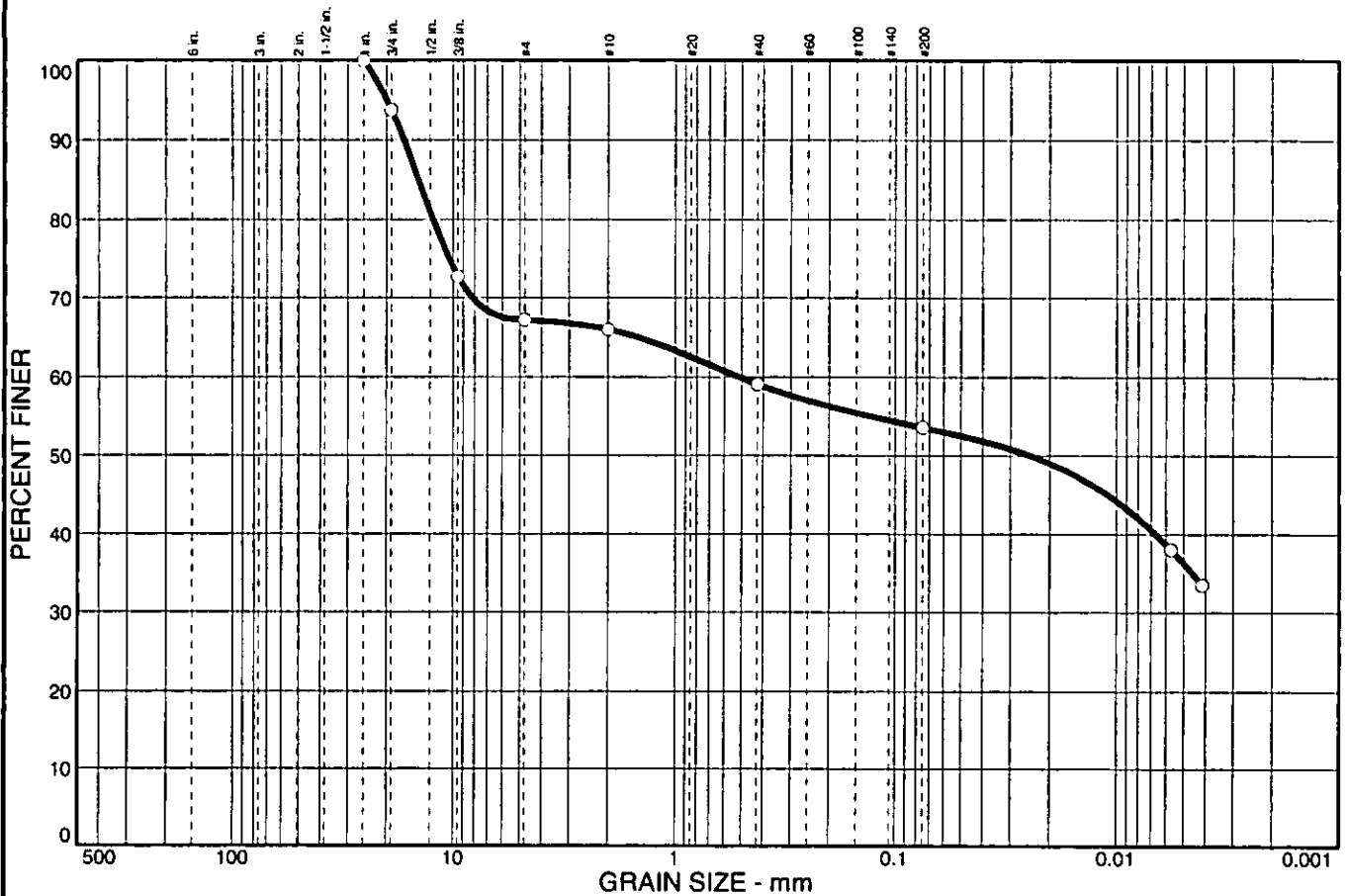
* (no specification provided)

Sample No.: 3 Source of Sample: R-360 Date: 2/16/05
Location: Elev./Depth: 6



Client: TranSystems, Inc.
Project: SCI-823-0.00
Project No: 0121-3070.03 Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	6.3	26.5	1.2	6.9	5.6	17.3	36.2

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.00 in.	100.0		
0.75 in.	93.7		
0.375 in.	72.6		
#4	67.2		
#10	66.0		
#40	59.1		
#200	53.5		

(no specification provided)

Soil Description

Gravelly lean clay

Atterberg Limits

PL= 23 LL= 40 PI= 17

Coefficients

D₈₅= 14.4 D₆₀= 0.515 D₅₀= 0.0247
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-6(7)

Remarks

Moisture Content= 12.0%
F.M.=0.67

Sample No.: 2
Location:

Source of Sample: R-365

Date: 2/16/05
Elev./Depth: 3.5

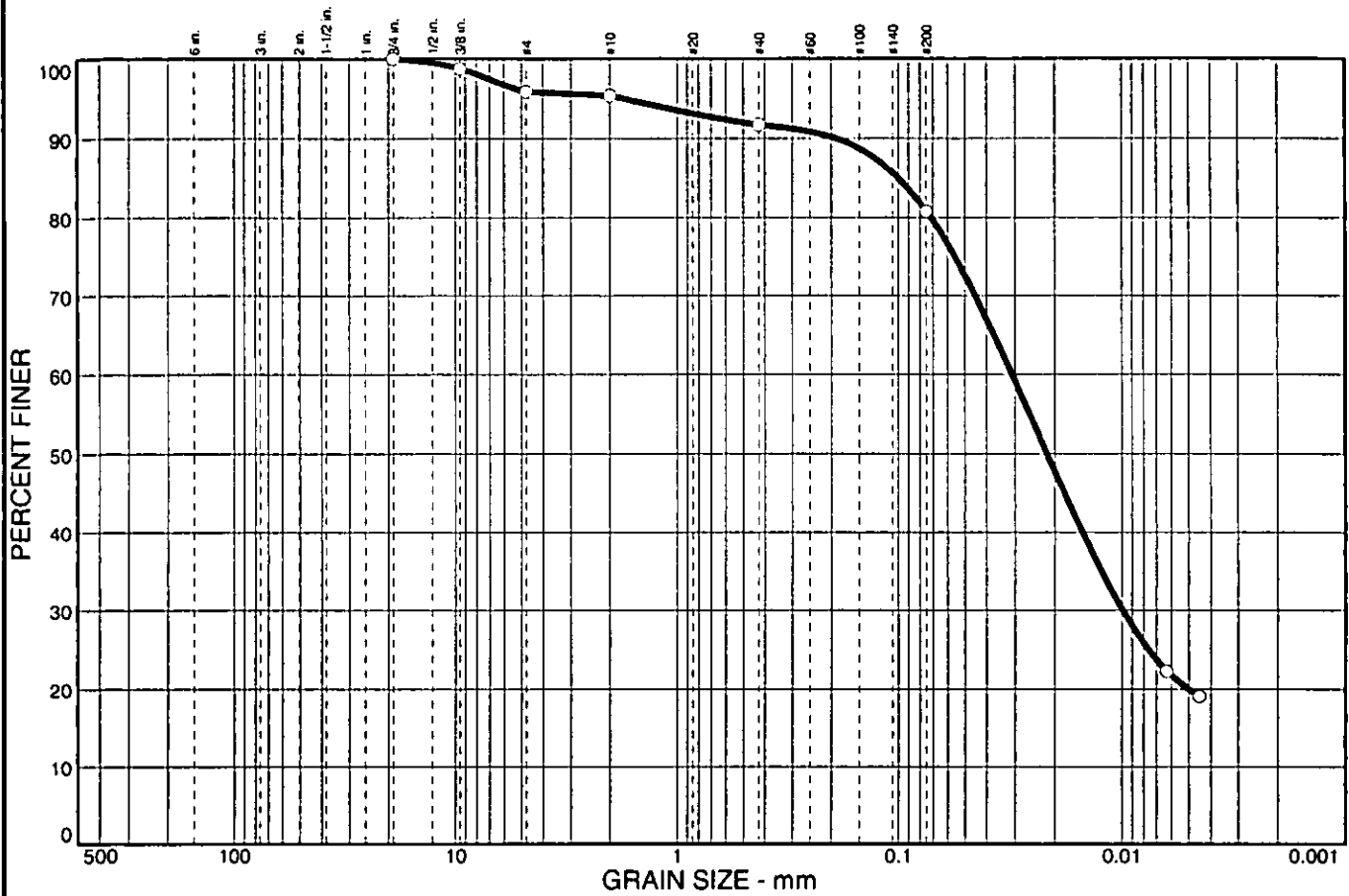


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	4.2	0.5	3.6	10.9	60.9	19.9

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	98.7		
#4	95.8		
#10	95.3		
#40	91.7		
#200	80.8		

Soil Description

Lean clay with sand

Atterberg Limits

PL= 17 LL= 25 PI= 8

Coefficients

D₈₅= 0.100 D₆₀= 0.0311 D₅₀= 0.0218
D₃₀= 0.0100 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(4)

Remarks

Moisture Content= 9.9%
F.M.=0.05

* (no specification provided)

Sample No.: 3
Location:

Source of Sample: R-367

Date: 3/10/05
Elev./Depth: 6

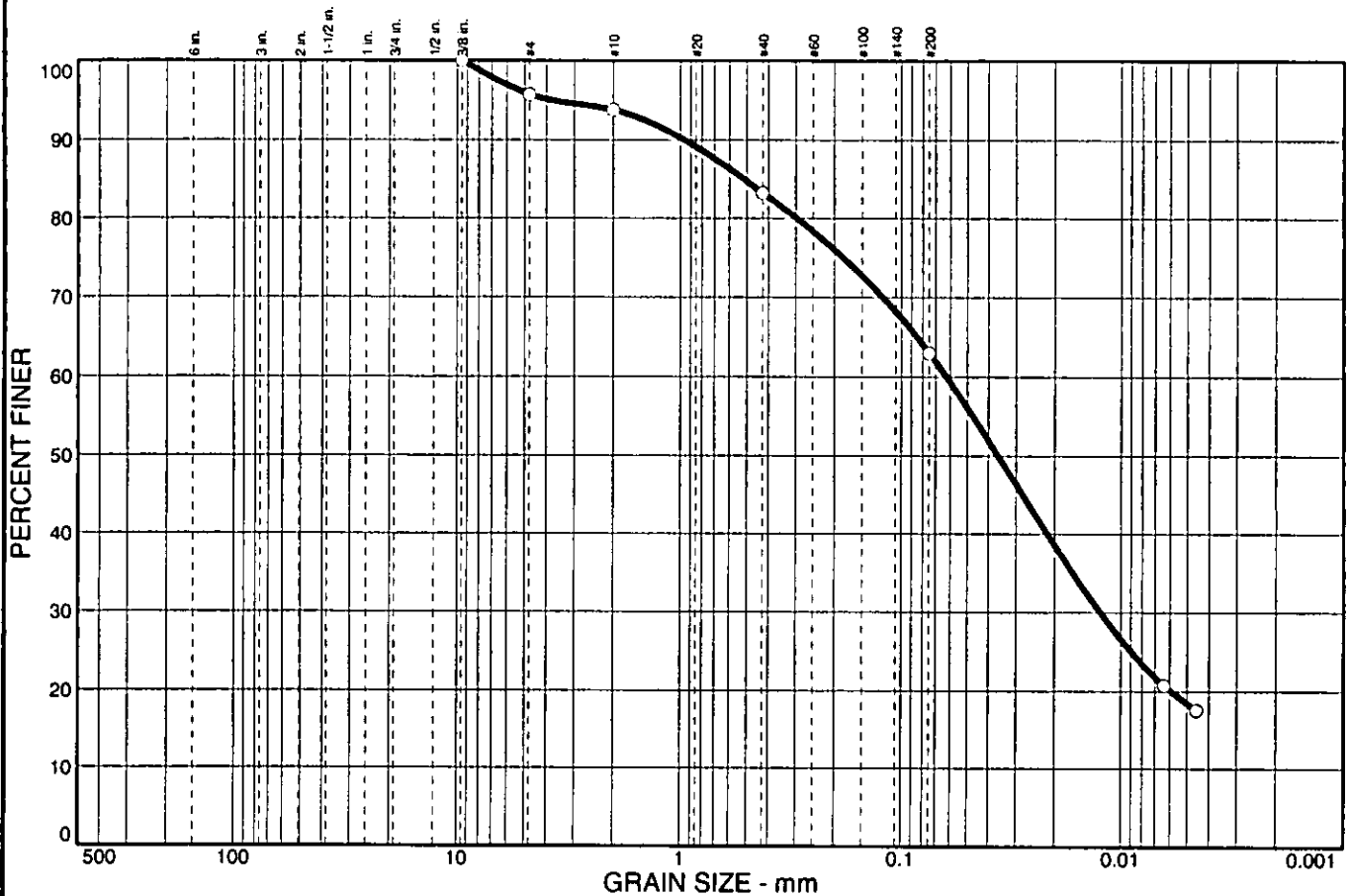


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	4.4	1.9	10.4	20.3	44.6	18.4

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	95.6		
#10	93.7		
#40	83.3		
#200	63.0		

(no specification provided)

Soil Description

Sandy silty clay

Atterberg Limits

PL= 17 LL= 23 PI= 6

Coefficients

D₈₅= 0.514 D₆₀= 0.0626 D₅₀= 0.0364
D₃₀= 0.0124 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(1)

Remarks

Moisture Content= 11.2%
F.M.=0.04

Sample No.: 2
Location:

Source of Sample: R-373

Date: 3/11/05
Elev./Depth: 3.5

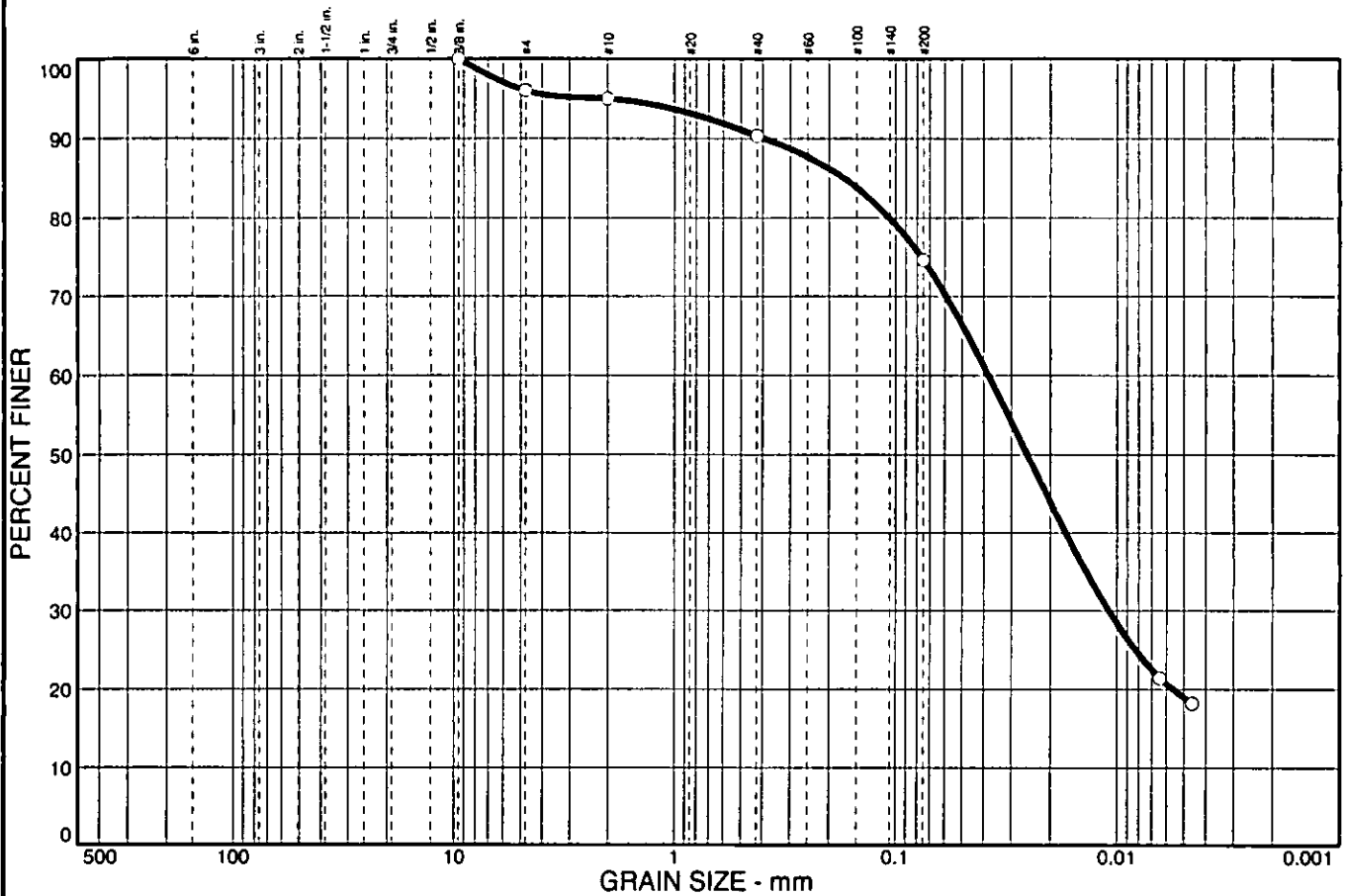


Client: TranSystems, Inc.
Project: SCI-823-0.00

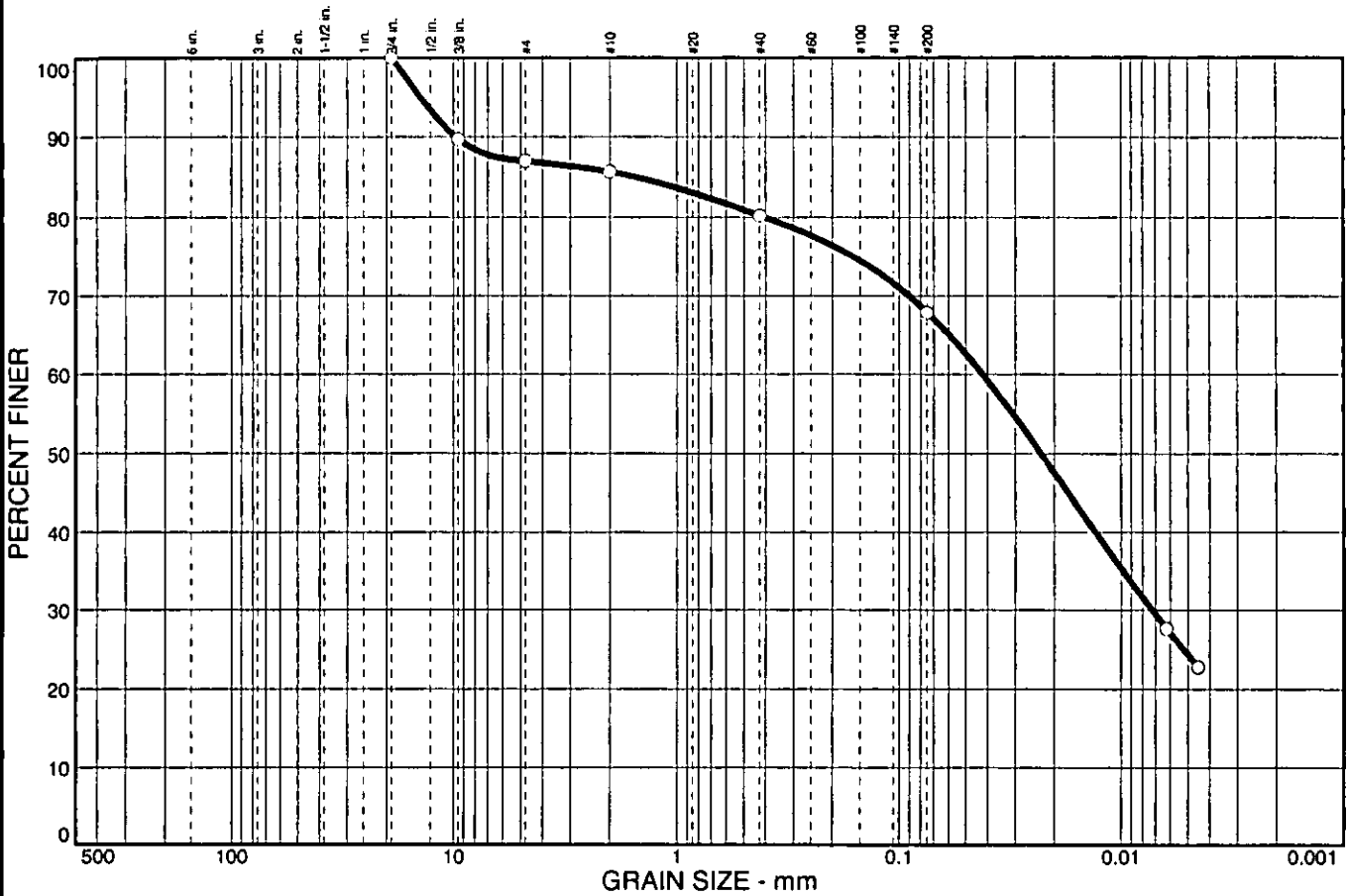
Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	13.0	1.3	5.5	12.3	43.6	24.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	89.7		
#4	87.0		
#10	85.7		
#40	80.2		
#200	67.9		

Soil Description

Sandy lean clay

Atterberg Limits

PL= 22 LL= 30 PI= 8

Coefficients

D₈₅= 1.51 D₆₀= 0.0422 D₅₀= 0.0231
D₃₀= 0.0073 C_c= D₁₀=

Classification

USCS= CL AASHTO= A-4(4)

Remarks

Moisture Content= 19.5%
F.M.=0.23

* (no specification provided)

Sample No.: IA
Location:

Source of Sample: R-379

Date: 2/7/05
Elev./Depth: 1.0

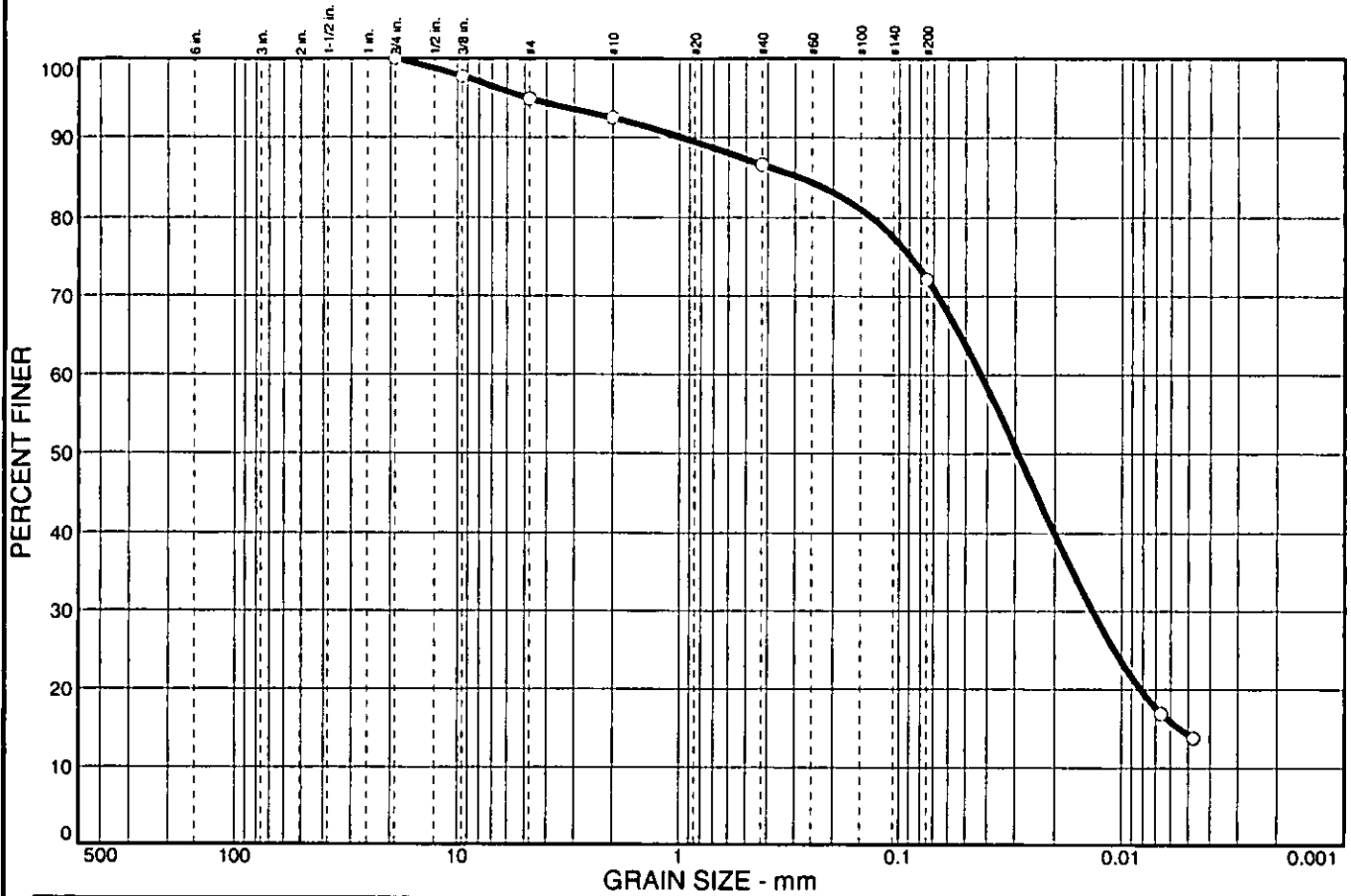


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	5.2	2.4	5.8	14.5	57.9	14.2

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	97.7		
#4	94.8		
#10	92.4		
#40	86.6		
#200	72.1		

Soil Description

Silt with sand

Atterberg Limits

PL= 22 LL= 27 PI= 5

Coefficients

D₈₅= 0.285 D₆₀= 0.0431 D₅₀= 0.0293
 D₃₀= 0.0136 D₁₅= 0.0055 D₁₀=
 C_u= C_c=

Classification

USCS= ML AASHTO= A-4(2)

Remarks

Moisture Content= 14.4%
 F.M.=0.08

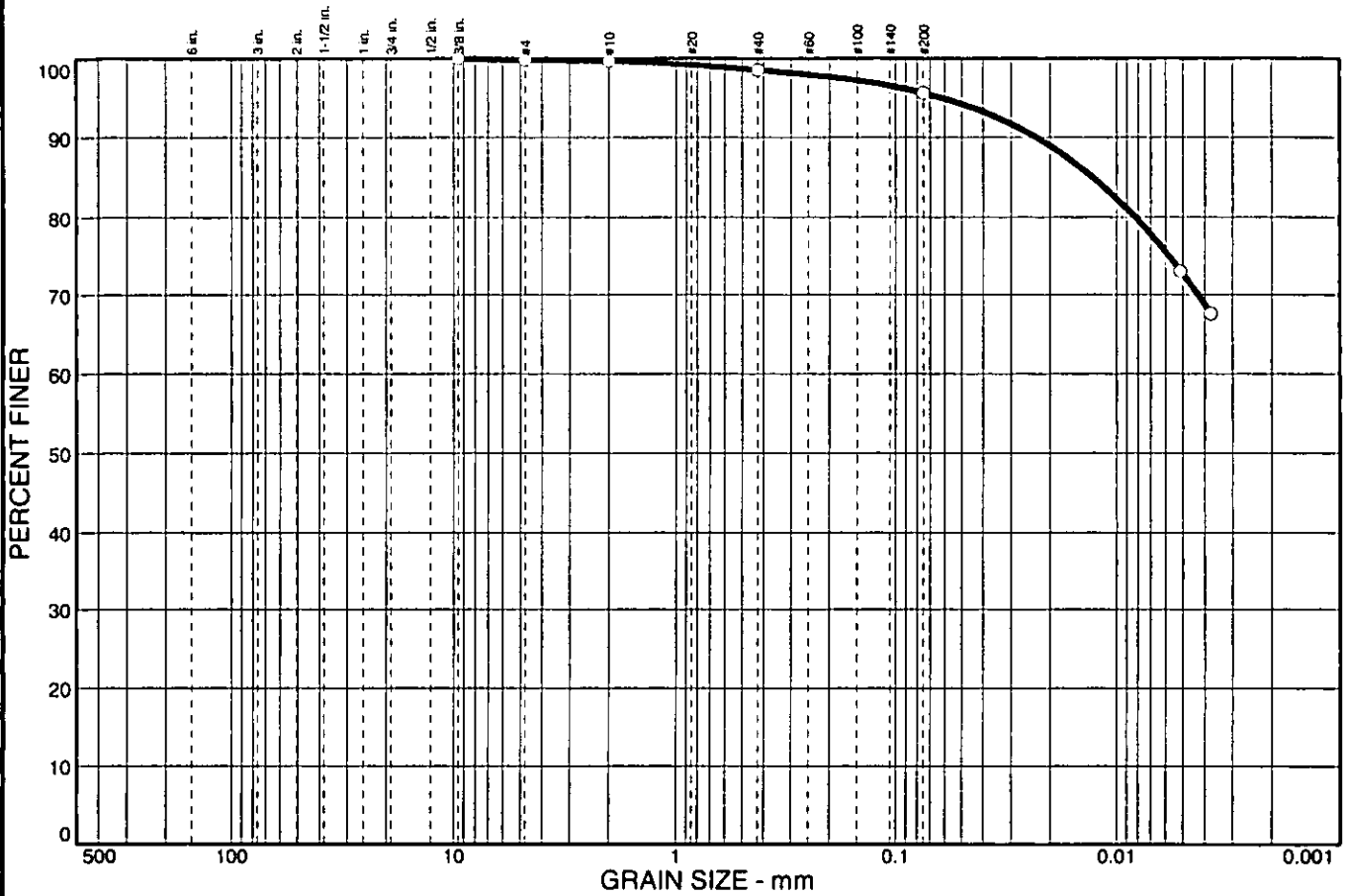
(no specification provided)

Sample No.: 1B Source of Sample: R-379 Date: 2/7/05
 Location: Elev./Depth: 2.0



Client: TranSystems, Inc.
 Project: SCI-823-0.00
 Project No: 0121-3070.03 Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.2	0.1	1.2	3.0	22.9	72.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.8		
#10	99.7		
#40	98.5		
#200	95.5		

Soil Description

Fat clay

Atterberg Limits

PL= 22 LL= 50 PI= 28

Coefficients

D₈₅= 0.0128 D₆₀= D₅₀=
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CH AASHTO= A-7-6(30)

Remarks

Moisture Content= 25.7%
F.M.=0.00

(no specification provided)

Sample No.: 5A
Location:

Source of Sample: R-379

Date: 2/7/05
Elev./Depth: 11.0

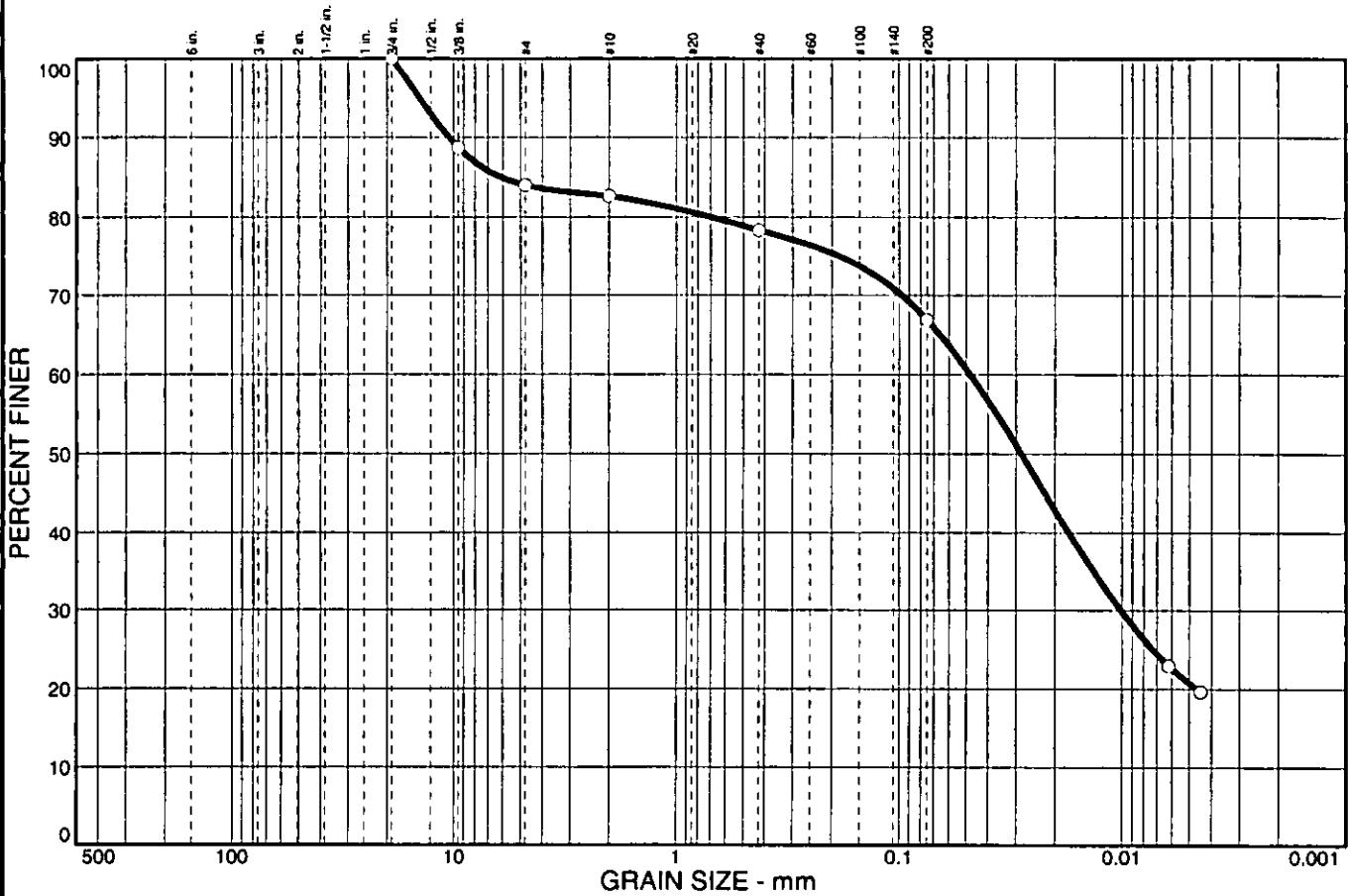


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	15.9	1.4	4.4	11.4	46.2	20.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	88.7		
#4	84.1		
#10	82.7		
#40	78.3		
#200	66.9		

* (no specification provided)

Soil Description

Sandy silty clay with gravel

Atterberg Limits

PL= 19 LL= 25 PI= 6

Coefficients

D₈₅= 6.00 D₆₀= 0.0482 D₅₀= 0.0285
D₃₀= 0.0102 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(2)

Remarks

Moisture Content= 14.3%
F.M.=0.27

Sample No.: 4
Location:

Source of Sample: R-380

Date: 3/11/05
Elev./Depth: 8.5

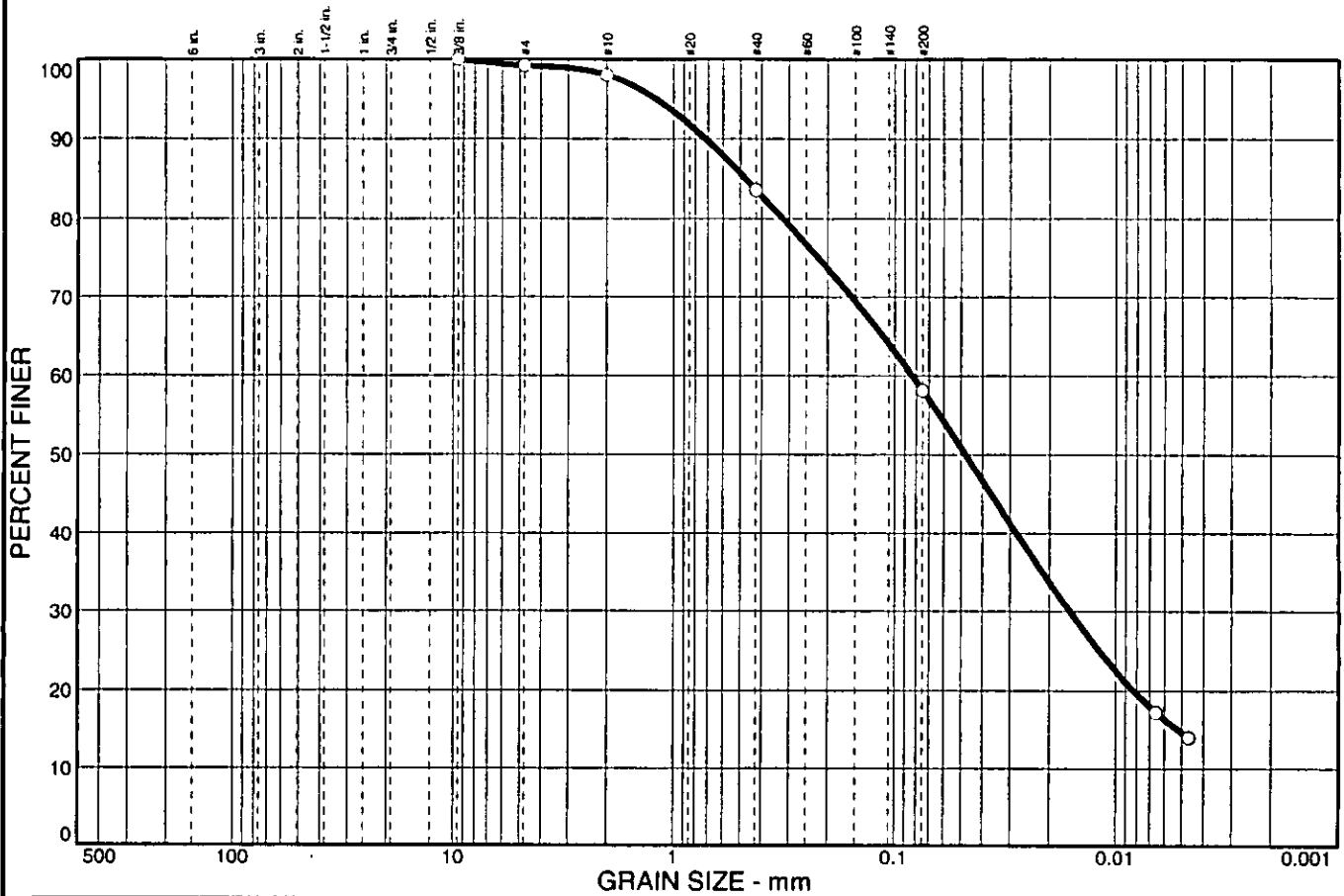


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.8	1.2	14.4	25.5	43.5	14.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.2		
#10	98.0		
#40	83.6		
#200	58.1		

Soil Description

Sandy silt

Atterberg Limits

PL= 19 LL= 21 PI= 2

Coefficients

D₈₅= 0.475 D₆₀= 0.0837 D₅₀= 0.0480
D₃₀= 0.0162 D₁₅= 0.0052 D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 11.6%
F.M.=0.01

(no specification provided)

Sample No.: 3
Location:

Source of Sample: R-392

Date: 3/10/05
Elev./Depth: 6

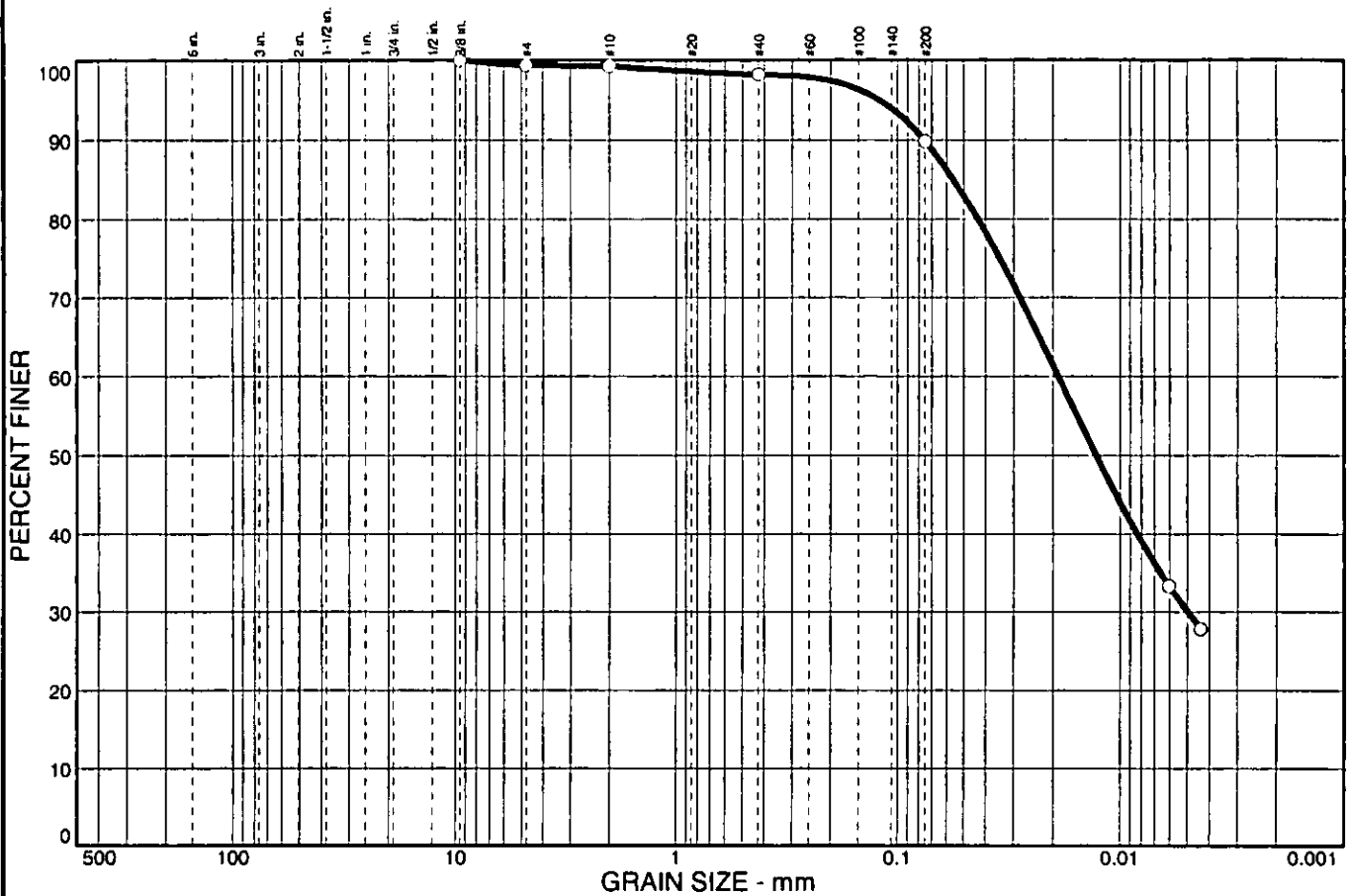


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.6	0.1	1.1	8.4	59.8	30.0

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.4		
#10	99.3		
#40	98.2		
#200	89.8		

Soil Description

Lean clay

Atterberg Limits

PL= 18 LL= 27 PI= 9

Coefficients

D₈₅= 0.0560 D₆₀= 0.0190 D₅₀= 0.0128
D₃₀= 0.0050 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(7)

Remarks

Moisture Content= 20.0%
F.M.=0.01

* (no specification provided)

Sample No.: 13
Location:

Source of Sample: R-393

Date: 2/16/05
Elev./Depth: 33.5

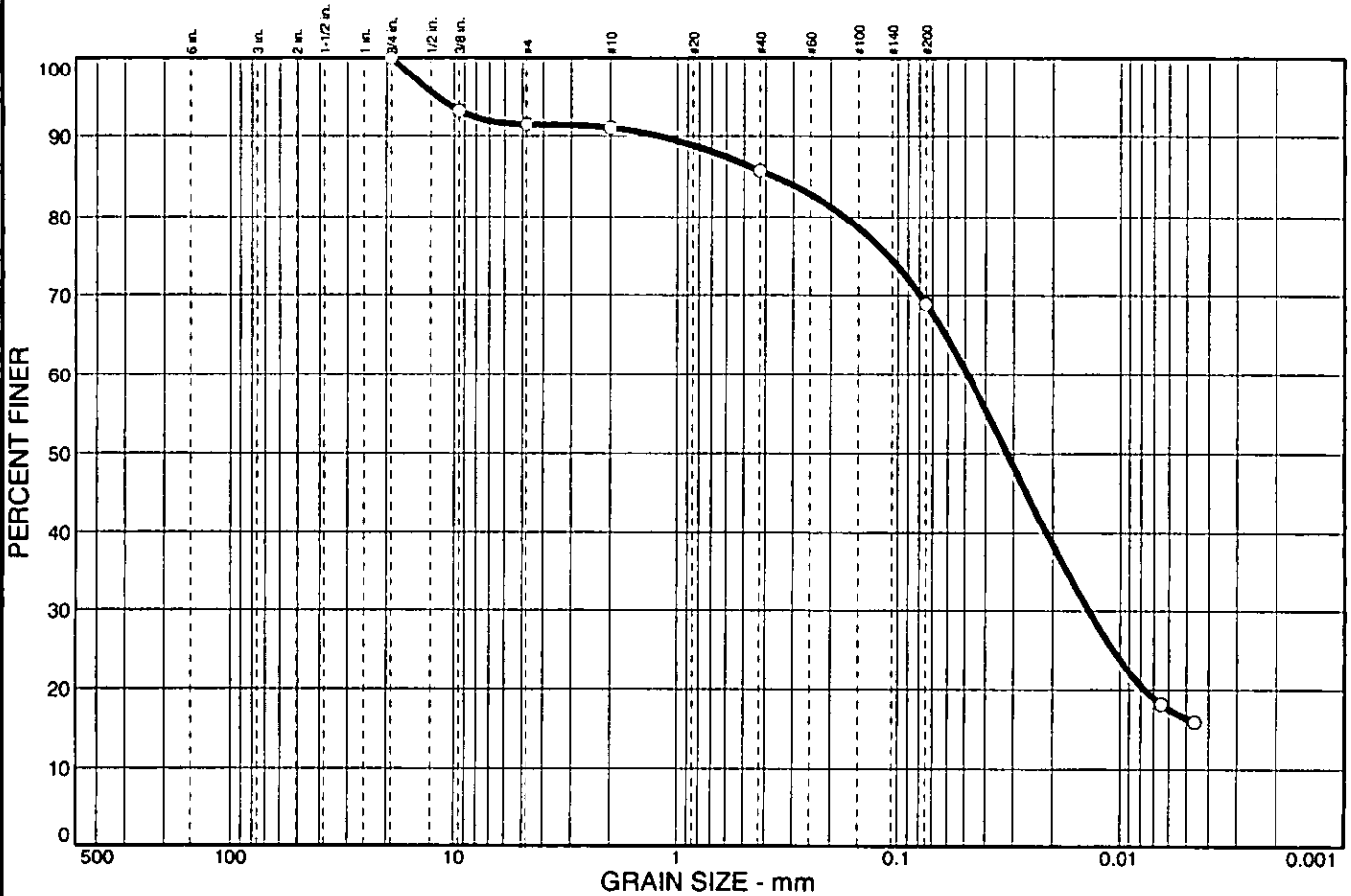


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	8.6	0.4	5.2	16.9	52.6	16.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	93.1		
#4	91.4		
#10	91.0		
#40	85.8		
#200	68.9		

Soil Description

Sandy silt

Atterberg Limits

PL= 19 LL= 22 PI= 3

Coefficients

D₈₅= 0.363 D₆₀= 0.0488 D₅₀= 0.0322
 D₃₀= 0.0139 D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 13.1%
 F.M.=0.16

(no specification provided)

Sample No.: 5
 Location:

Source of Sample: R-394

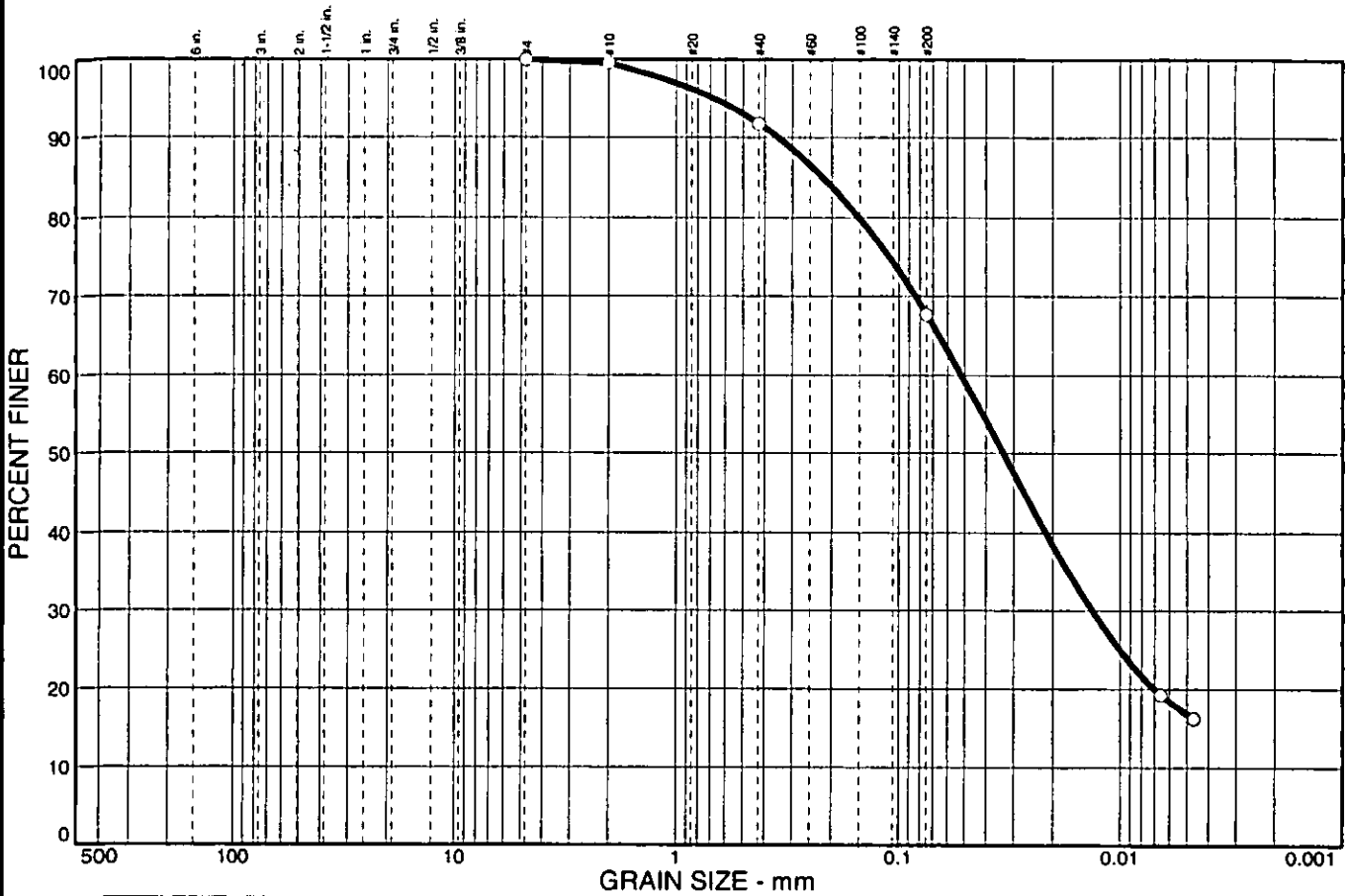
Date: 3/10/05
 Elev./Depth: 11



Client: TranSystems, Inc.
 Project: SCI-823-0.00
 Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.4	7.9	24.0	50.9	16.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.6		
#40	91.7		
#200	67.7		

Soil Description

Sandy silt

Atterberg Limits

PL= 22 LL= 24 PI= 2

Coefficients

D₈₅= 0.221 D₆₀= 0.0521 D₅₀= 0.0335
D₃₀= 0.0133 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 15.8%

* (no specification provided)

Sample No.: 4
Location:

Source of Sample: R-395

Date: 3/10/05
Elev./Depth: 8.5

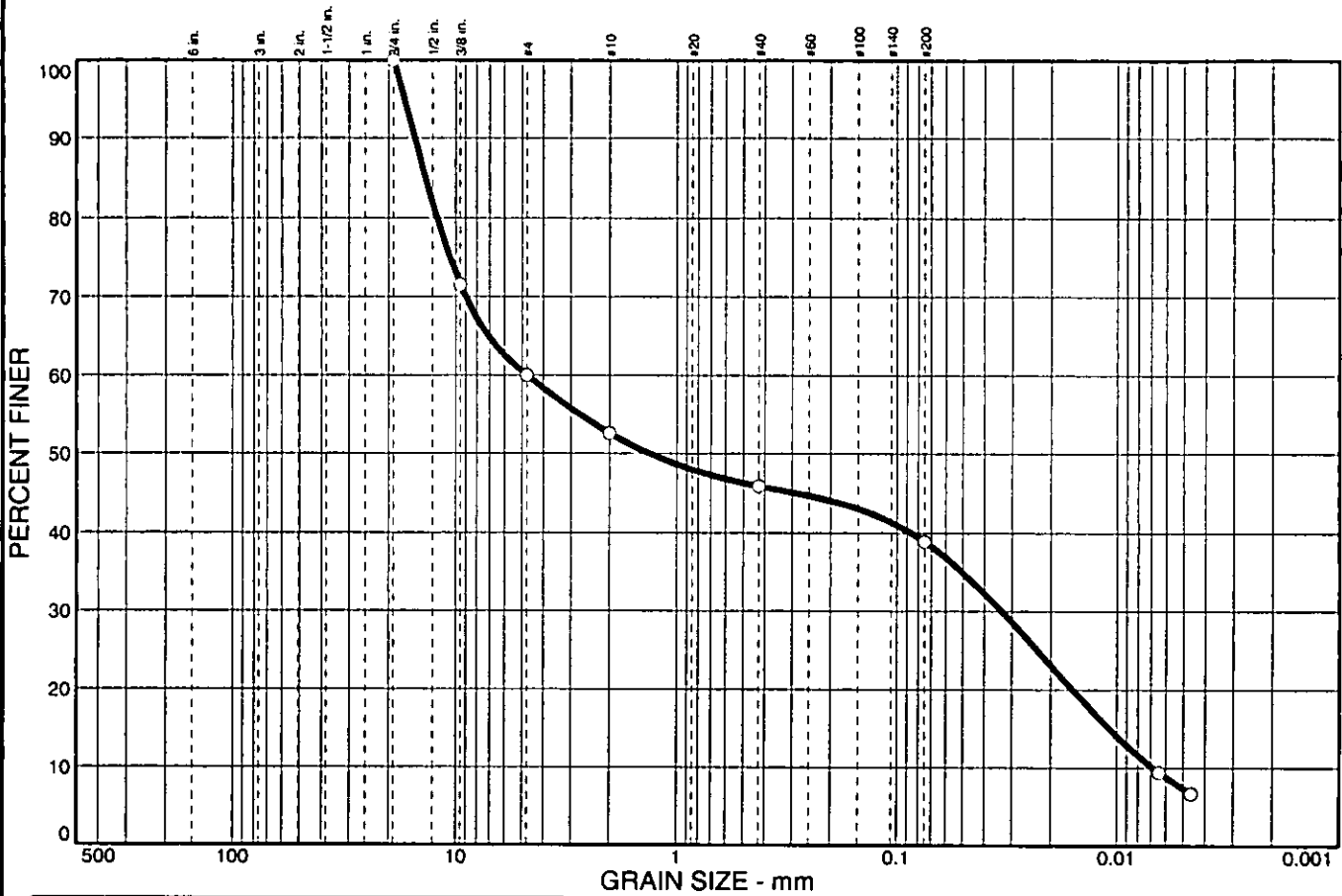


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	40.0	7.5	6.7	6.9	31.6	7.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	71.5		
#4	60.0		
#10	52.5		
#40	45.8		
#200	38.9		

Soil Description

Silty gravel with sand

Atterberg Limits

PL= 20 LL= 23 PI= 3

Coefficients

D₈₅= 13.7 D₆₀= 4.75 D₅₀= 1.34
D₃₀= 0.0335 D₁₅= 0.0108 D₁₀= 0.0069
C_u= 692.24 C_c= 0.03

Classification

USCS= GM AASHTO= A-4(0)

Remarks

Moisture Content= 12.0%
F.M.=0.69

(no specification provided)

Sample No.: 2
Location:

Source of Sample: R-396

Date: 2/16/05
Elev./Depth: 3.5

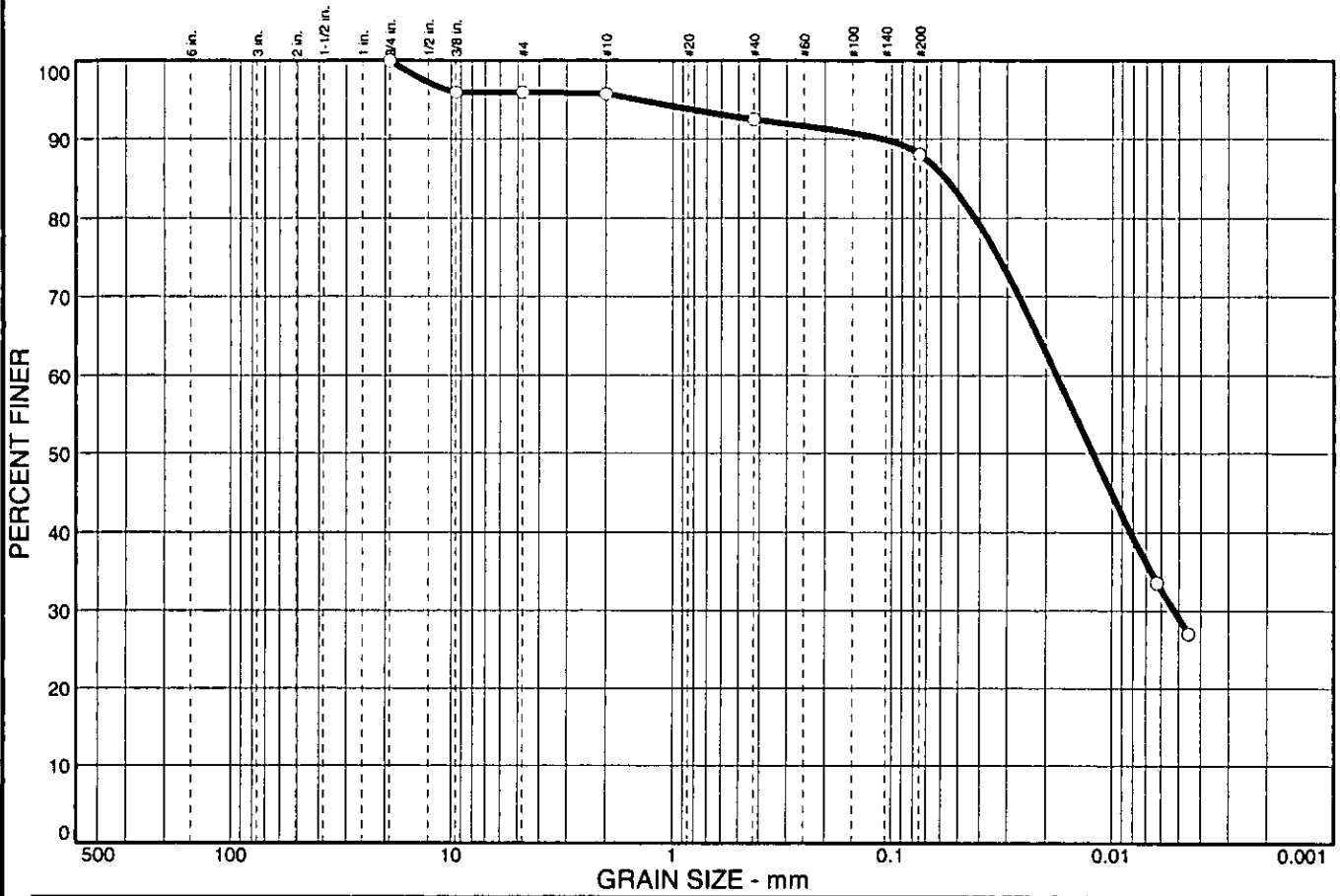


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	4.1	0.2	3.3	4.4	59.0	29.0

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	95.9		
#4	95.9		
#10	95.7		
#40	92.4		
#200	88.0		

* (no specification provided)

Soil Description

Lean clay

Atterberg Limits

PL= 21 LL= 37 PI= 16

Coefficients

D₈₅= 0.0573 D₆₀= 0.0179 D₅₀= 0.0123
 D₃₀= 0.0053 D₁₅= D₁₀=
 C_u= C_c=

Classification

USCS= CL AASHTO= A-6(14)

Remarks

Moisture Content= 22.2%
 F.M.=0.08

Sample No.: 6
Location:

Source of Sample: R-396

Date: 2/7/05
Elev./Depth: 13.5

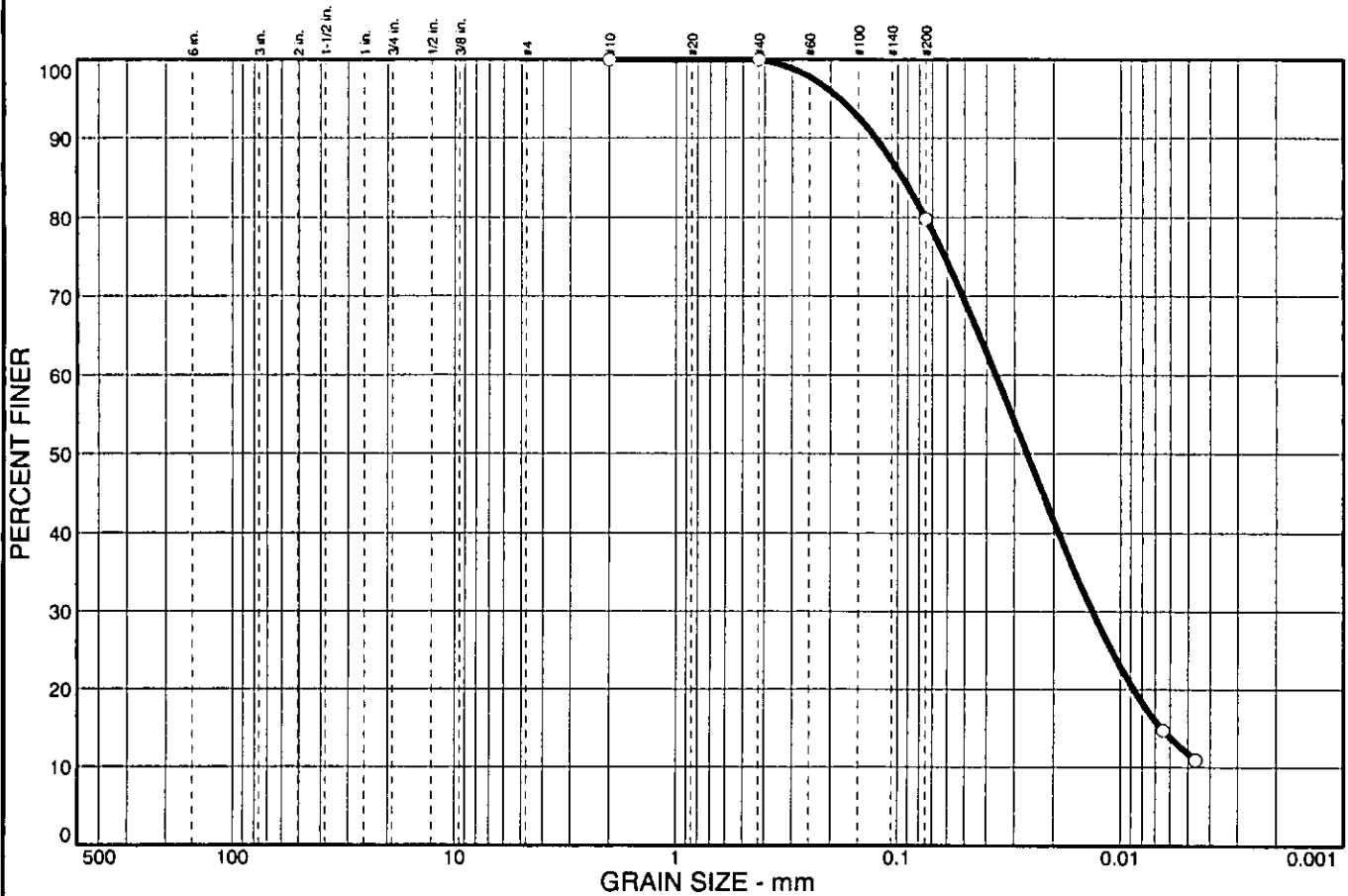


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.0	20.2	68.1	11.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	100.0		
#200	79.8		

Soil Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 0.0954 D₆₀= 0.0366 D₅₀= 0.0264
 D₃₀= 0.0134 D₁₅= 0.0066 D₁₀=
 C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 21.9%

* (no specification provided)

Sample No.: 9
Location:

Source of Sample: R-396

Date: 2/16/05
Elev./Depth: 21

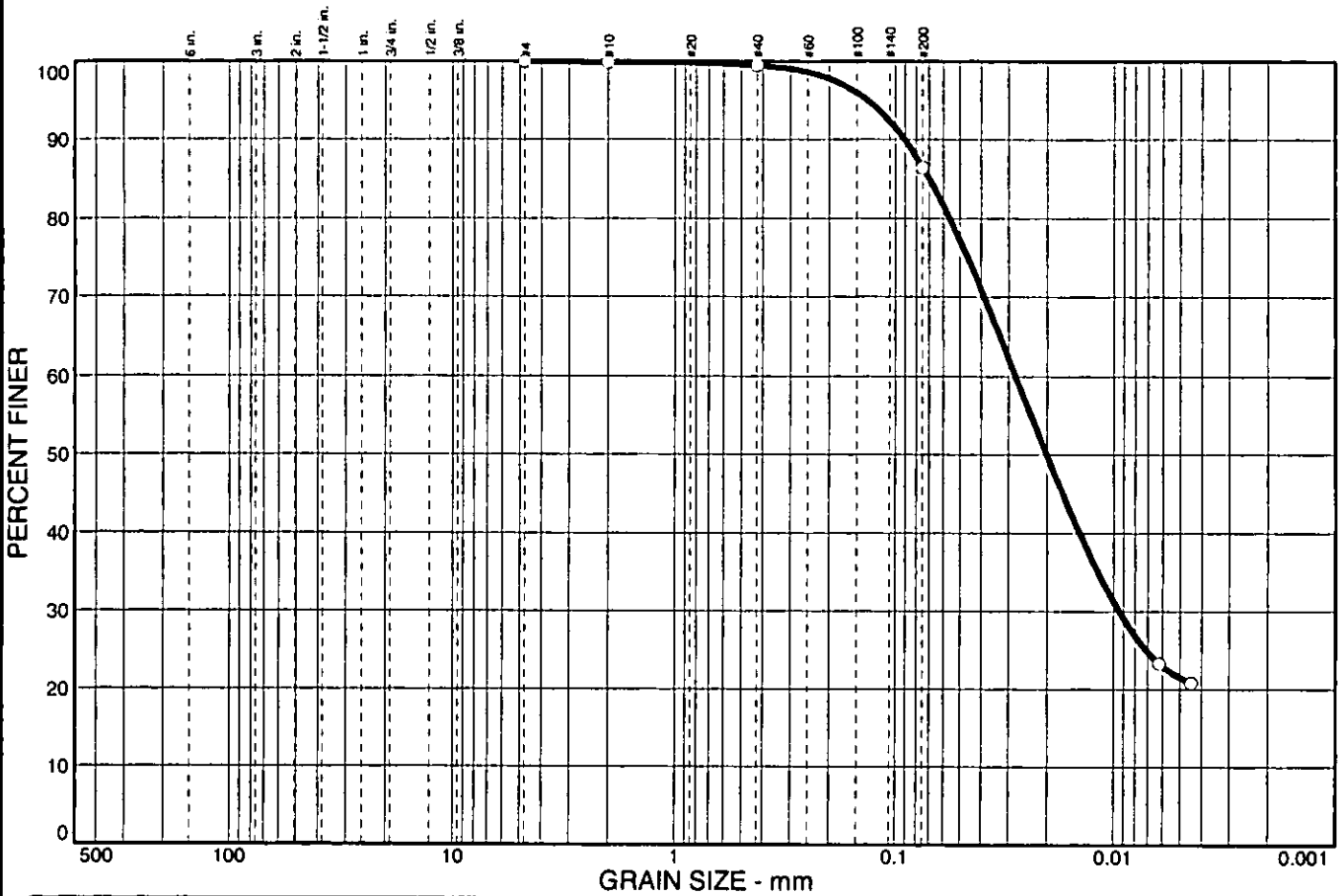


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.1	0.4	13.0	65.0	21.5

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#4	100.0		
#10	99.9		
#40	99.5		
#200	86.5		

* (no specification provided)

Soil Description

Silt

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D₈₅= 0.0697 D₆₀= 0.0280 D₅₀= 0.0202
D₃₀= 0.0095 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Moisture Content= 22.5%

Sample No.: 2
Location:

Source of Sample: R-399

Date: 2/16/05
Elev./Depth: 3.5

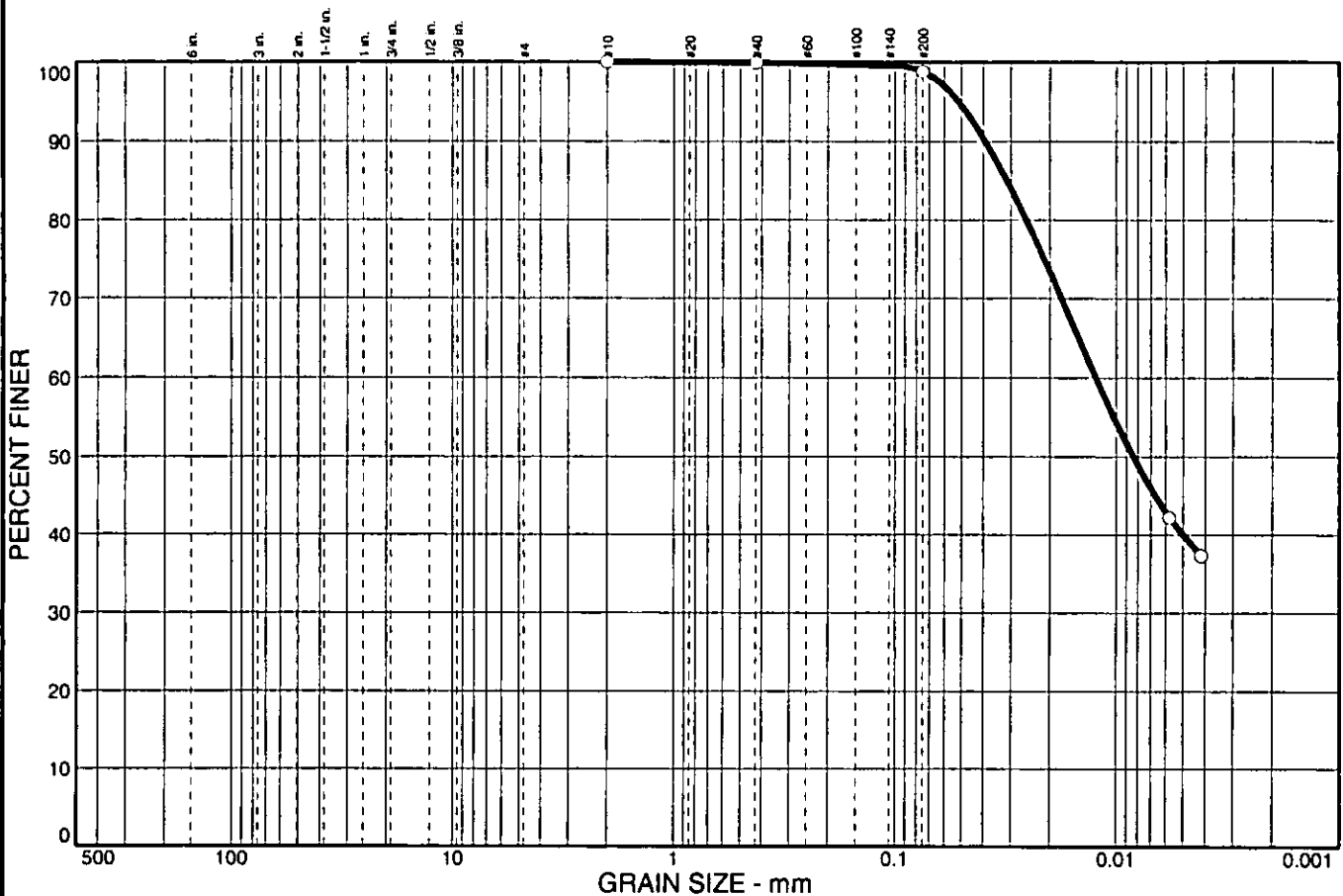


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.1	1.1	58.9	39.9

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.9		
#200	98.8		

Soil Description

Lean clay

Atterberg Limits

PL= 20 LL= 40 PI= 20

Coefficients

D₈₅= 0.0314 D₆₀= 0.0124 D₅₀= 0.0084
D₃₀= D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-6(21)

Remarks

Moisture Content= 16.7%

* (no specification provided)

Sample No.: 4
Location:

Source of Sample: R-399

Date: 2/16/05
Elev./Depth: 8.5

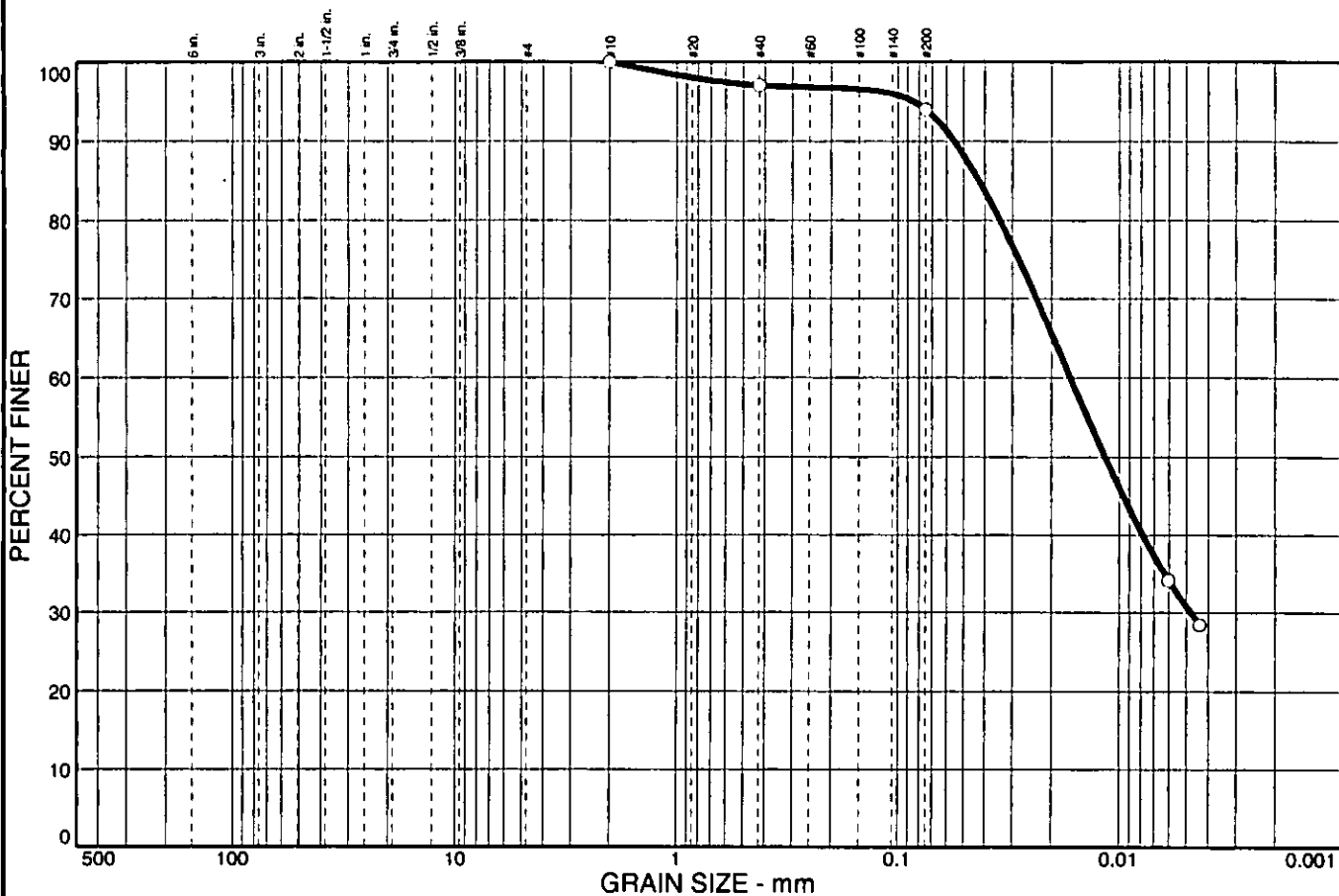


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	3.0	3.1	63.2	30.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	97.0		
#200	93.9		

* (no specification provided)

Soil Description

Lean clay

Atterberg Limits

PL= 21 LL= 30 PI= 9

Coefficients

D₈₅= 0.0428 D₆₀= 0.0165 D₅₀= 0.0116
D₃₀= 0.0048 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(8)

Remarks

Moisture Content= 19.3%

Sample No.: 1
Location:

Source of Sample: R-402

Date: 3/10/05
Elev./Depth: 1.0

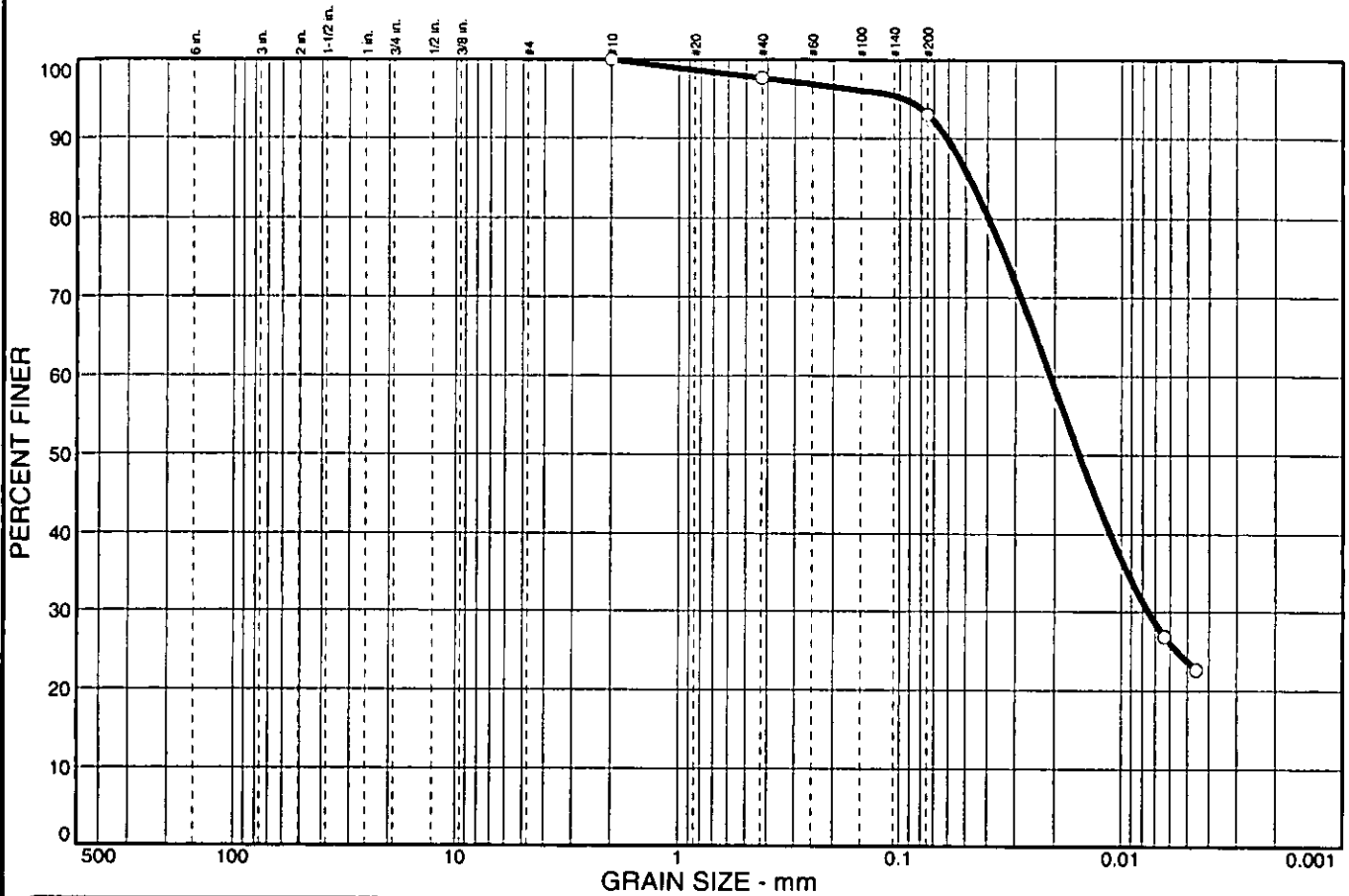


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	2.4	4.6	69.4	23.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	97.6		
#200	93.0		

* (no specification provided)

Soil Description

Lean clay

Atterberg Limits

PL= 22 LL= 30 PI= 8

Coefficients

D₈₅= 0.0483 D₆₀= 0.0210 D₅₀= 0.0155
D₃₀= 0.0076 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(7)

Remarks

Moisture Content= 19.7%

Sample No.: 2
Location:

Source of Sample: R-402

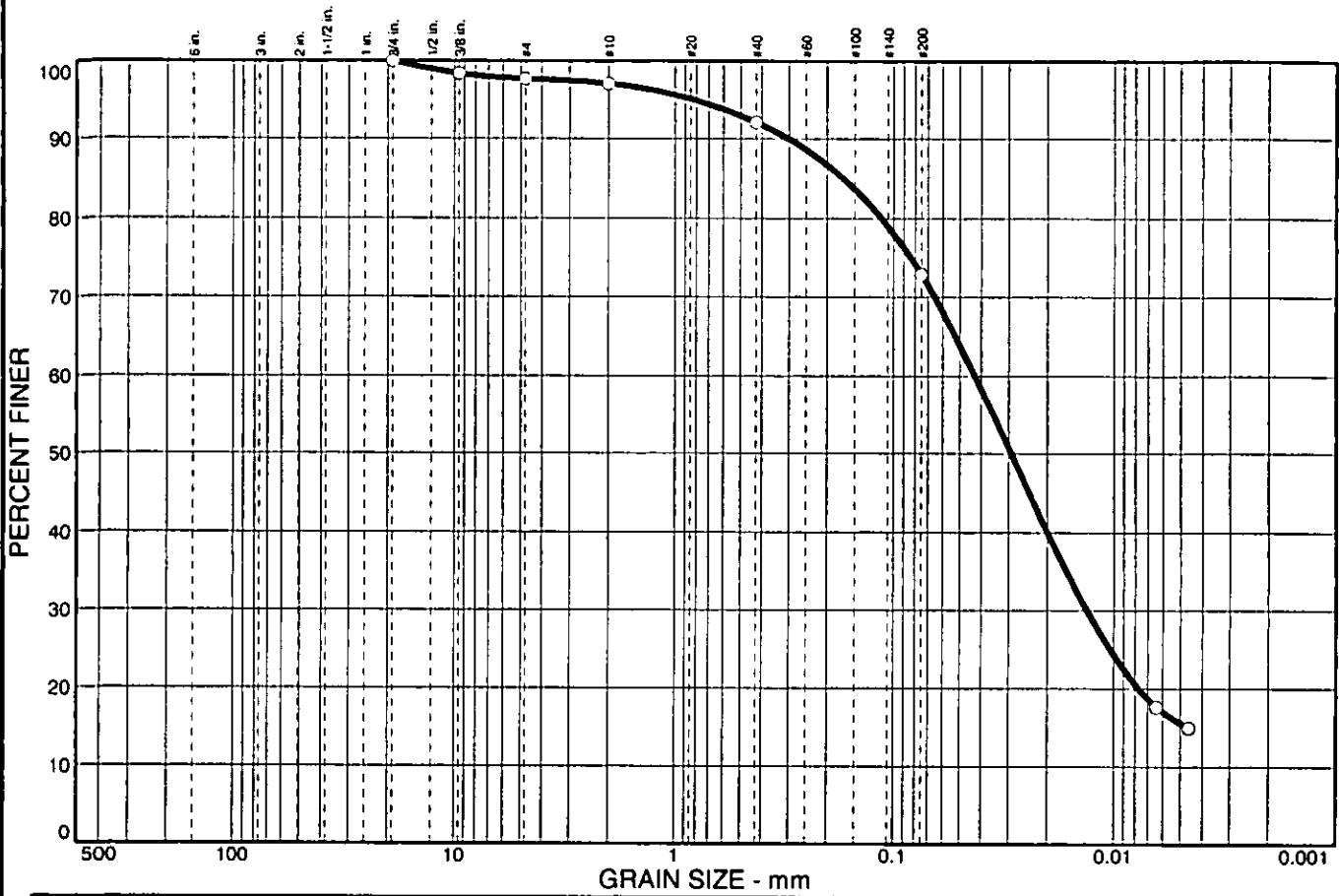
Date: 3/10/05
Elev./Depth: 3.5



Client: TranSystems, Inc.
Project: SCI-823-0.00
Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	2.3	0.6	5.1	19.2	57.3	15.5

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	98.4		
#4	97.7		
#10	97.1		
#40	92.0		
#200	72.8		

Soil Description

Silt with sand

Atterberg Limits

PL= 22 LL= 25 PI= 3

Coefficients

D₈₅= 0.170 D₆₀= 0.0430 D₅₀= 0.0293
 D₃₀= 0.0133 D₁₅= 0.0046 D₁₀=
 C_u=

Classification

USCS= ML AASHTO= A-4(1)

Remarks

Moisture Content= 17.7%
 F.M.=0.04

* (no specification provided)

Sample No.: 5
 Location:

Source of Sample: R-402

Date: 3/10/05
 Elev./Depth: 11

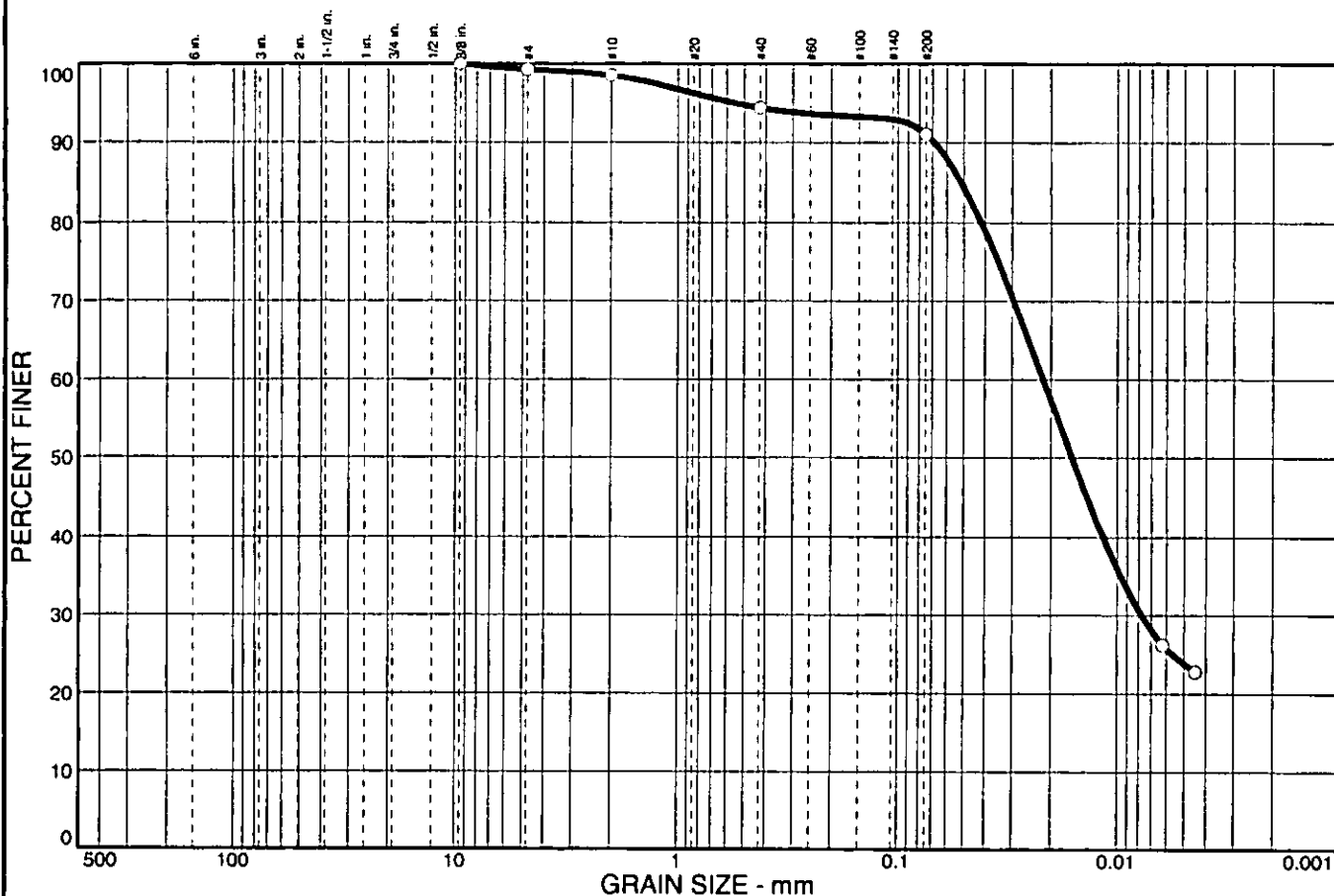


Client: TranSystems, Inc.
 Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.7	0.8	4.2	3.3	67.2	23.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.3		
#10	98.5		
#40	94.3		
#200	91.0		

* (no specification provided)

Soil Description

Lean clay

Atterberg Limits

PL= 21 LL= 32 PI= 11

Coefficients

D₈₅= 0.0517 D₆₀= 0.0218 D₅₀= 0.0161
D₃₀= 0.0078 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-6(10)

Remarks

Moisture Content= 20.6%
F.M.=0.01

Sample No.: 4
Location:

Source of Sample: R-405

Date: 3/30/05
Elev./Depth: 8.5



DLZ

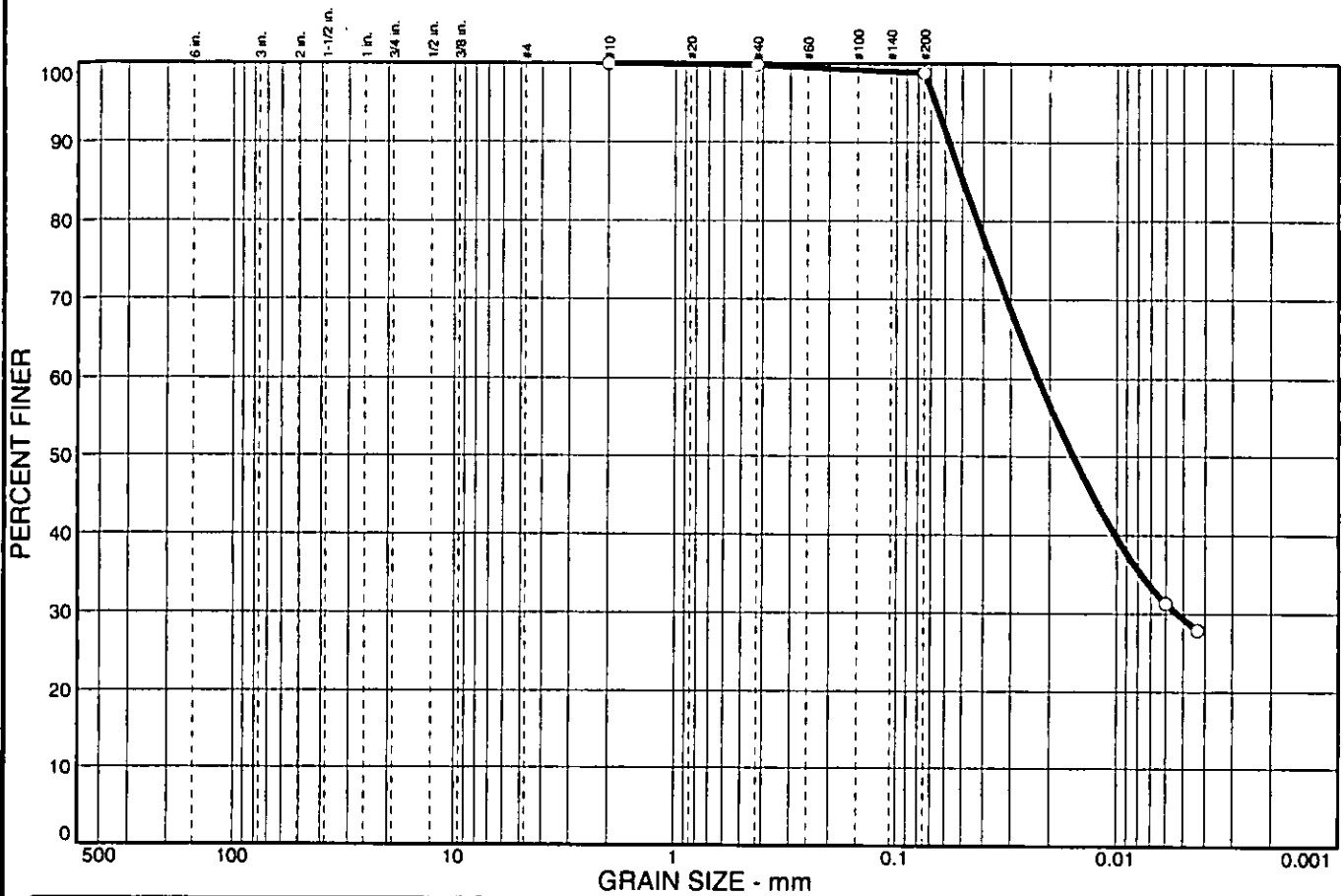
Client: TranSystems, Inc.

Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.2	1.0	69.5	29.3

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.8		
#200	98.8		

Soil Description

Lean clay

Atterberg Limits

PL= 20 LL= 33 PI= 13

Coefficients

D₈₅= 0.0499 D₆₀= 0.0226 D₅₀= 0.0157
D₃₀= 0.0053 C_u= D₁₅= D₁₀=
C_c=

Classification

USCS= CL AASHTO= A-6(13)

Remarks

Moisture content= 23.8%

(no specification provided)

Sample No.: 5
Location:

Source of Sample: R-405

Date: 3/30/05
Elev./Depth: 11

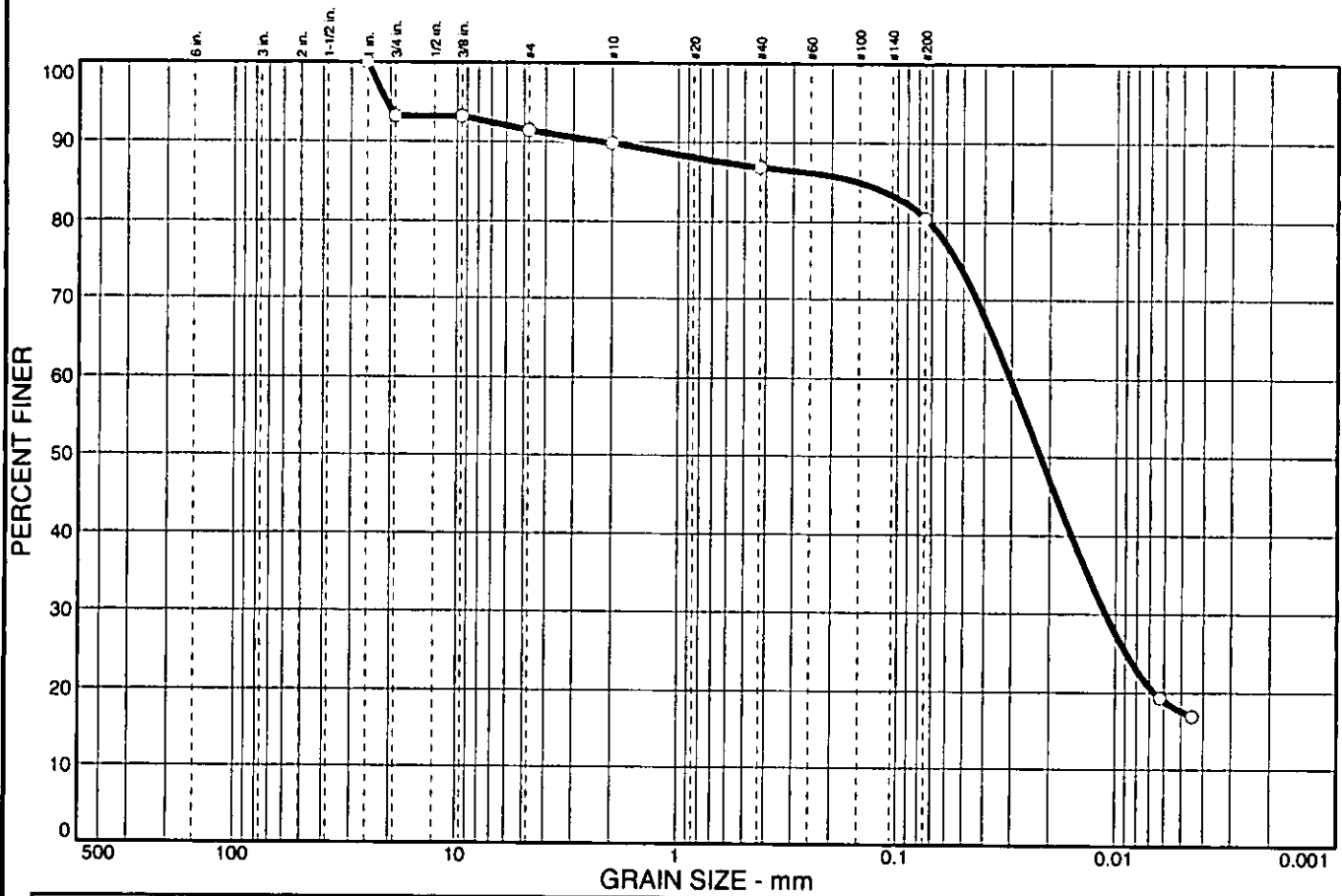


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	6.9	1.7	1.6	2.9	6.4	63.0	17.5

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.00 in.	100.0		
0.75 in.	93.1		
0.375 in.	93.1		
#4	91.4		
#10	89.8		
#40	86.9		
#200	80.5		

Soil Description

Silty clay with sand

Atterberg Limits

PL= 19 LL= 25 PI= 6

Coefficients

D₈₅= 0.149 D₆₀= 0.0306 D₅₀= 0.0221
D₃₀= 0.0111 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(3)

Remarks

Moisture Content= 16.1%
F.M.=0.22

* (no specification provided)

Sample No.: 1
Location:

Source of Sample: R-408

Date: 2/16/05
Elev./Depth: 1.0

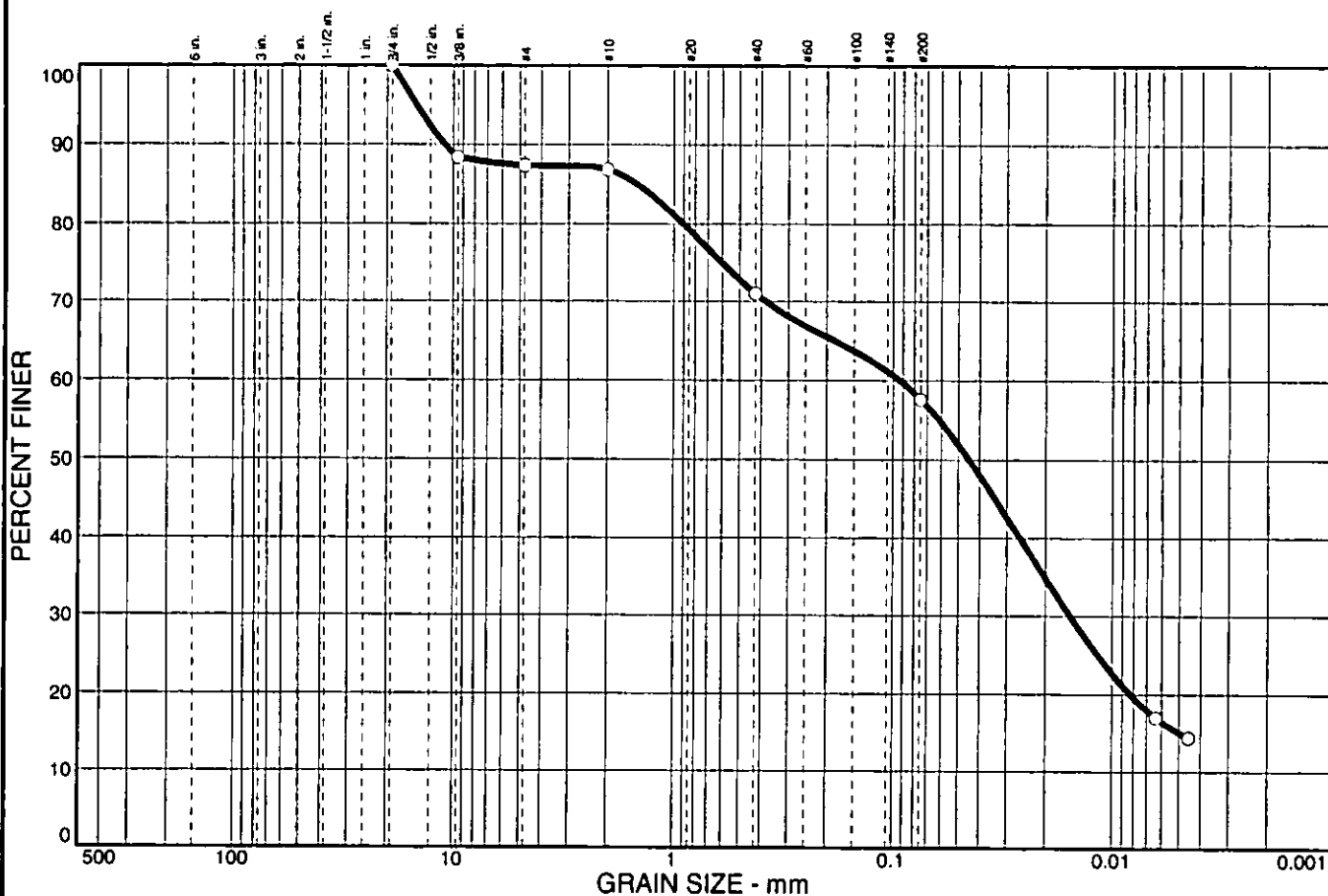


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	12.6	0.5	15.9	13.4	42.5	15.1

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.75 in.	100.0		
0.375 in.	88.4		
#4	87.4		
#10	86.9		
#40	71.0		
#200	57.6		

Soil Description

Sandy silty clay

Atterberg Limits

PL= 22 LL= 28 PI= 6

Coefficients

D₈₅= 1.46 D₆₀= 0.0935 D₅₀= 0.0451
D₃₀= 0.0155 D₁₅= 0.0049 D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(2)

Remarks

Moisture Content= 14.4%
F.M.=0.24

(no specification provided)

Sample No.: 3
Location:

Source of Sample: R-409

Date: 3/30/05
Elev./Depth: 6



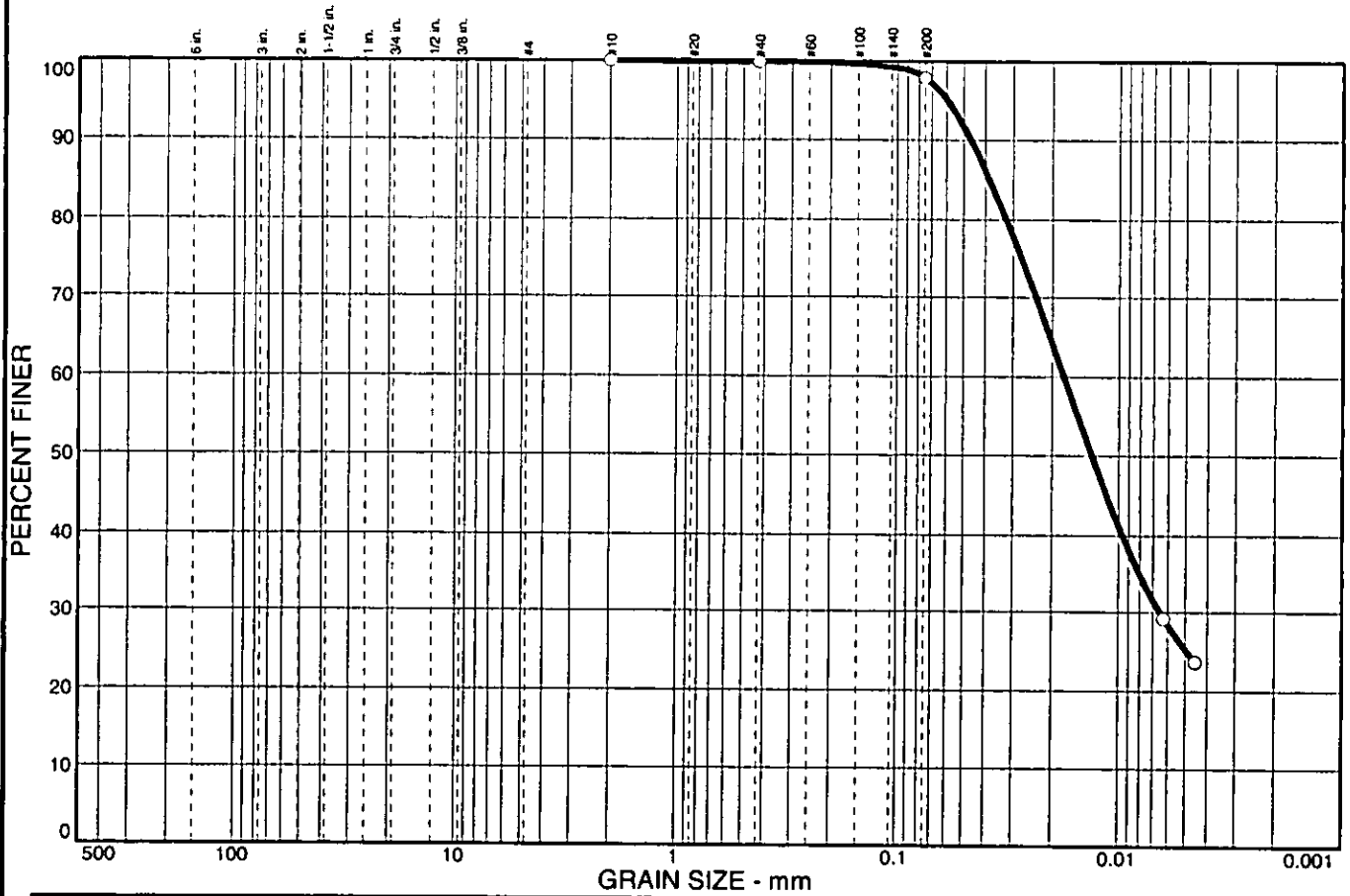
DLZ

Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.1	2.1	72.6	25.2

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.9		
#200	97.8		

Soil Description

Lean clay

Atterberg Limits

PL= 18 LL= 27 PI= 9

Coefficients

D₈₅= 0.0384 D₆₀= 0.0178 D₅₀= 0.0133
D₃₀= 0.0065 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-4(8)

Remarks

Moisture Content= 18.4%

* (no specification provided)

Sample No.: 6
Location:

Source of Sample: R-409

Date: 2/7/05
Elev./Depth: 13.5

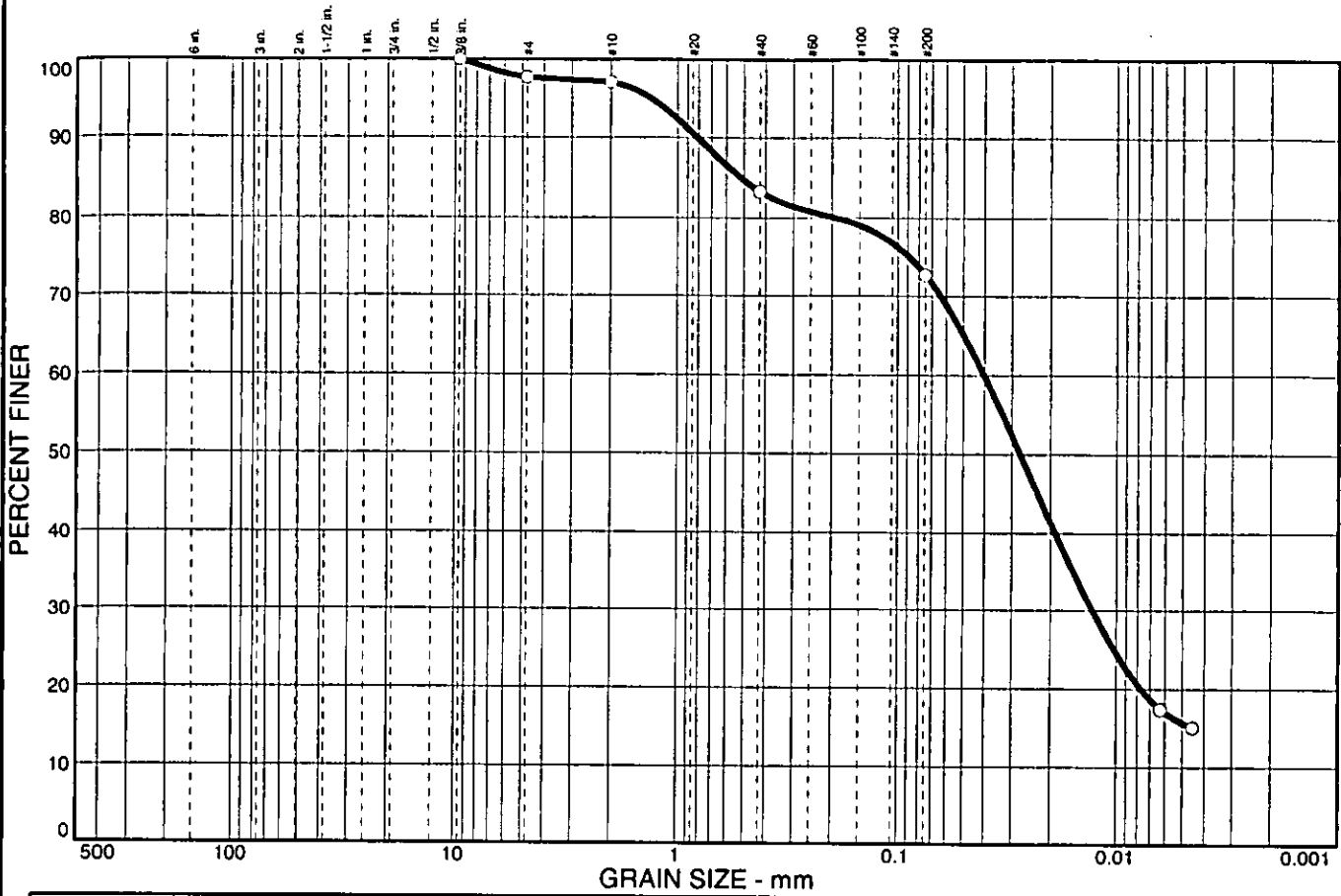


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	2.4	0.6	13.6	10.7	57.0	15.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	97.6		
#10	97.0		
#40	83.4		
#200	72.7		

Soil Description

Silty clay with sand

Atterberg Limits

PL= 21 LL= 26 PI= 5

Coefficients

D₈₅= 0.513 D₆₀= 0.0404 D₅₀= 0.0276
D₃₀= 0.0129 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL-ML AASHTO= A-4(2)

Remarks

Moisture Content= 13.3%
F.M.=0.02

* (no specification provided)

Sample No.: 10
Location:

Source of Sample: R-410

Date: 3/30/05
Elev./Depth: 23.5

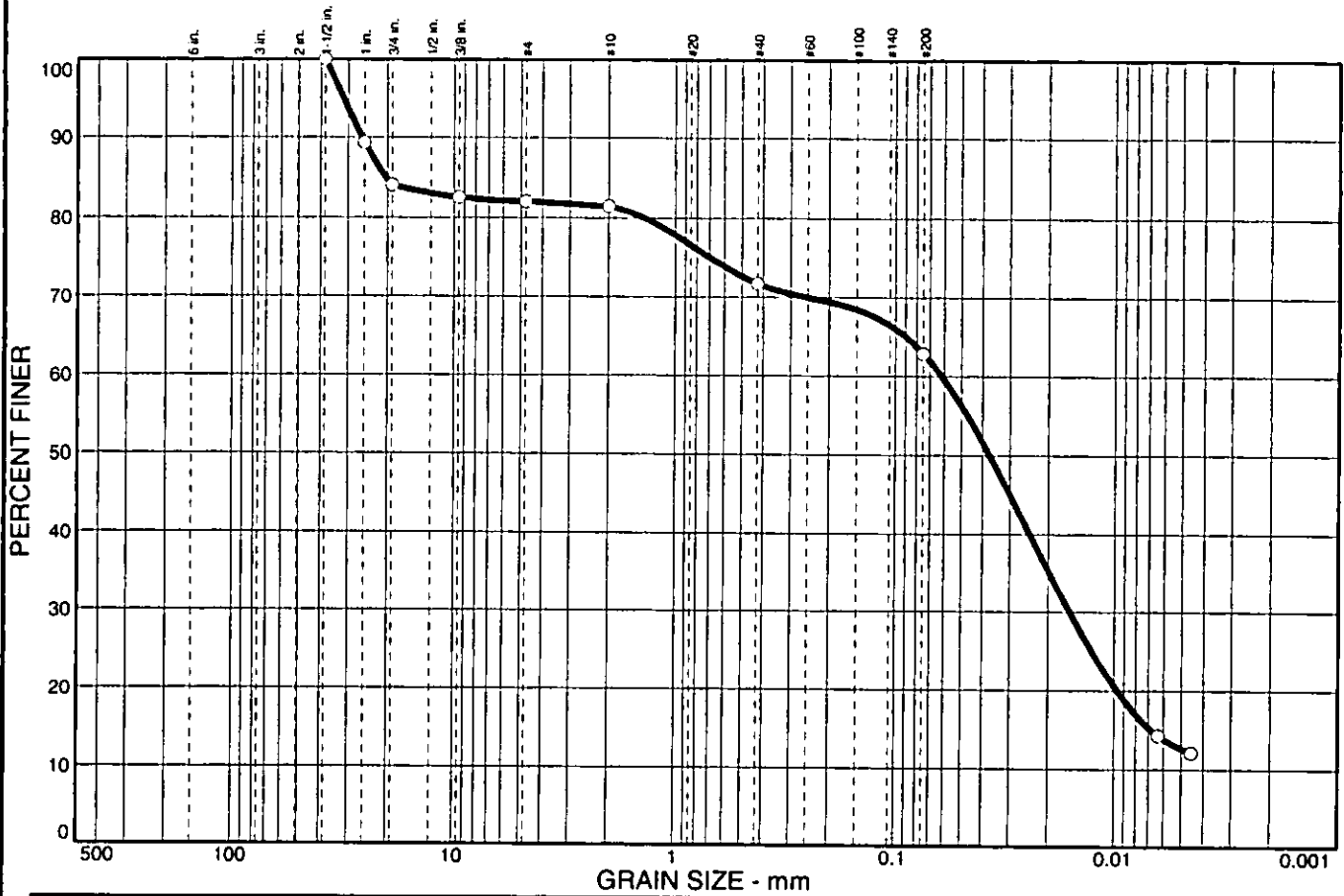


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	15.9	2.0	0.6	9.8	8.8	50.3	12.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1.50 in.	100.0		
1.00 in.	89.3		
0.75 in.	84.1		
0.375 in.	82.6		
#4	82.1		
#10	81.5		
#40	71.7		
#200	62.9		

Soil Description

Sandy silt with gravel

Atterberg Limits

PL= 22 LL= 26 PI= 4

Coefficients

D₈₅= 20.3 D₆₀= 0.0618 D₅₀= 0.0374
D₃₀= 0.0160 D₁₅= 0.0068 D₁₀=
C_u= C_c=

Classification

USCS= ML AASHTO= A-4(1)

Remarks

Moisture Content= 12.6%
F.M.=0.51

* (no specification provided)

Sample No.: 2
Location:

Source of Sample: R-410

Date: 3/30/05
Elev./Depth: 3.5

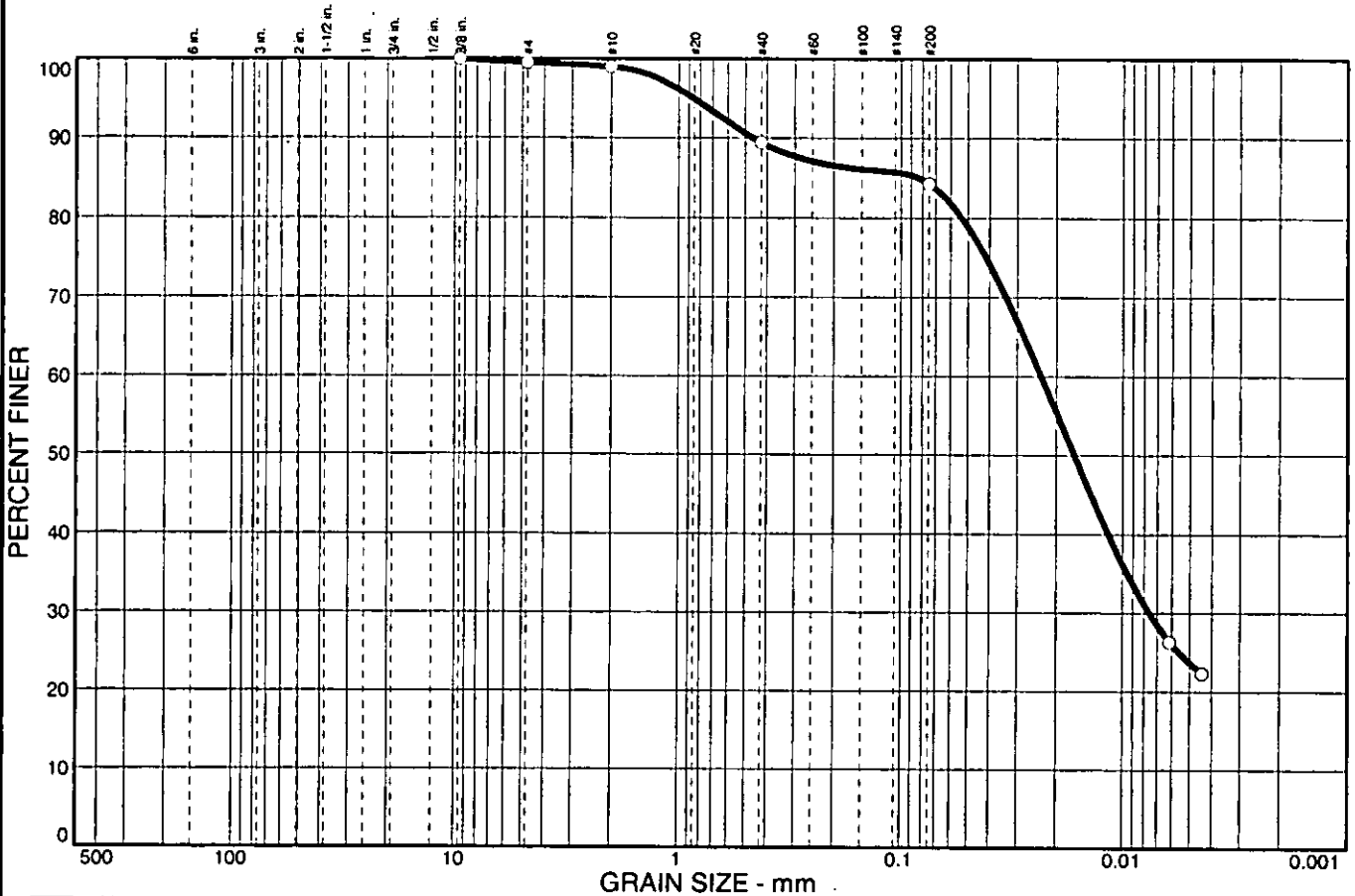


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.5	0.5	9.6	5.1	60.6	23.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
0.375 in.	100.0		
#4	99.5		
#10	99.0		
#40	89.4		
#200	84.3		

(no specification provided)

Soil Description

Lean clay with sand

Atterberg Limits

PL= 20 LL= 32 PI= 12

Coefficients

D₈₅= 0.0833 D₆₀= 0.0235 D₅₀= 0.0167
D₃₀= 0.0076 D₁₅= D₁₀=
C_u= C_c=

Classification

USCS= CL AASHTO= A-6(9)

Remarks

Moisture Content= 19.6%
F.M.=0.00

Sample No.: 5
Location:

Source of Sample: R-410

Date: 3/30/05
Elev./Depth: 11

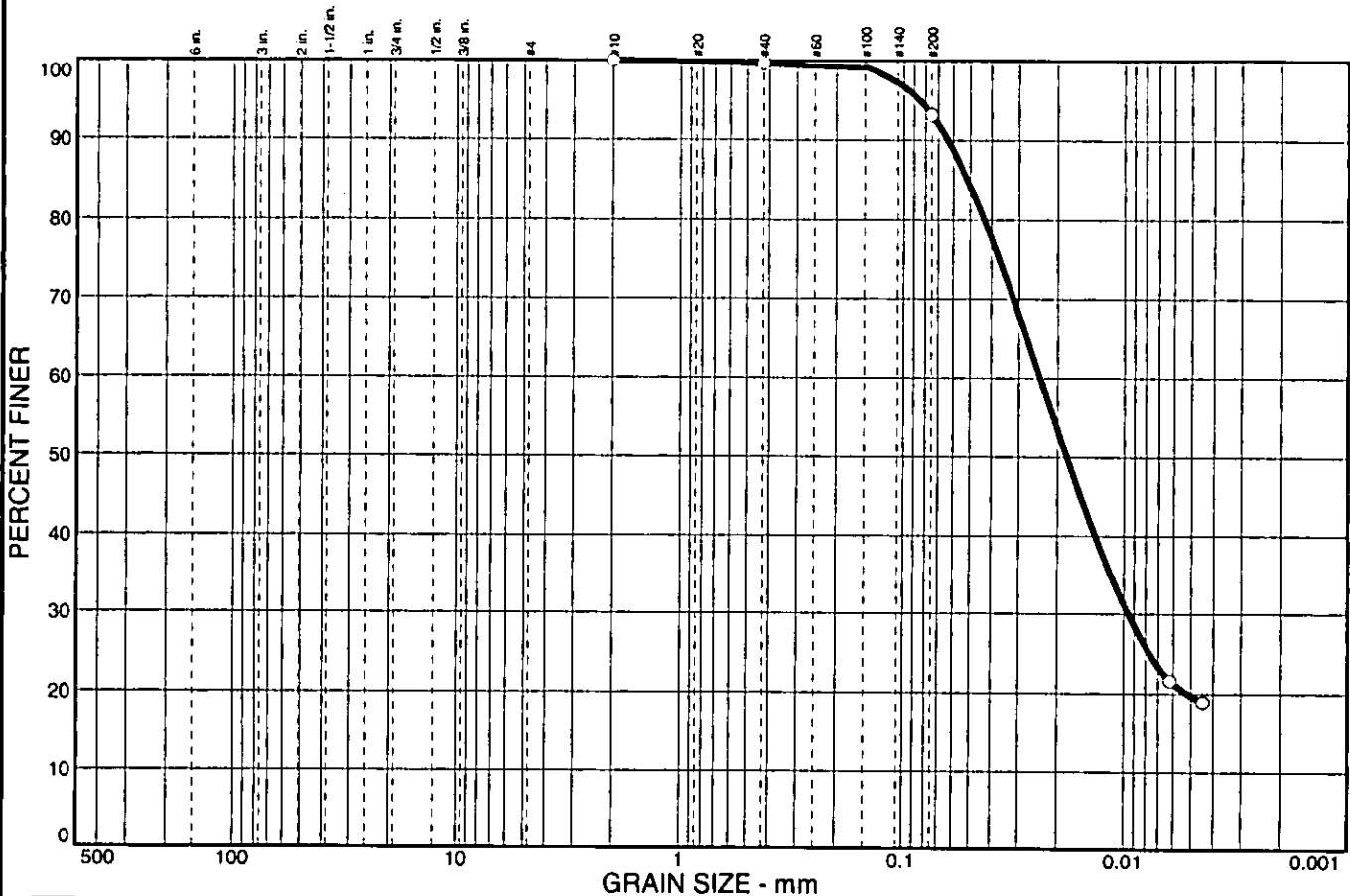


Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No.: 0121-3070.03

Figure

PARTICLE SIZE DISTRIBUTION TEST REPORT



% COBBLES	% GRAVEL		% SAND			% FINES	
	CRS.	FINE	CRS.	MEDIUM	FINE	SILT	CLAY
0.0	0.0	0.0	0.0	0.4	6.5	73.4	19.7

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#40	99.6		
#200	93.1		

(no specification provided)

Soil Description

Silt

Atterberg Limits
 PL= 24 LL= 32 PI= 8

Coefficients
 D₈₅= 0.0517 D₆₀= 0.0241 D₅₀= 0.0182
 D₃₀= 0.0096 D₁₅= D₁₀=
 C_u= C_c=

Classification
 USCS= ML AASHTO= A-4(8)

Remarks
 Moisture Content= 21.4%

Sample No.: 2
Location:

Source of Sample: R-424

Date: 3/30/05
Elev./Depth: 3.5



Client: TranSystems, Inc.
Project: SCI-823-0.00

Project No.: 0121-3070.03

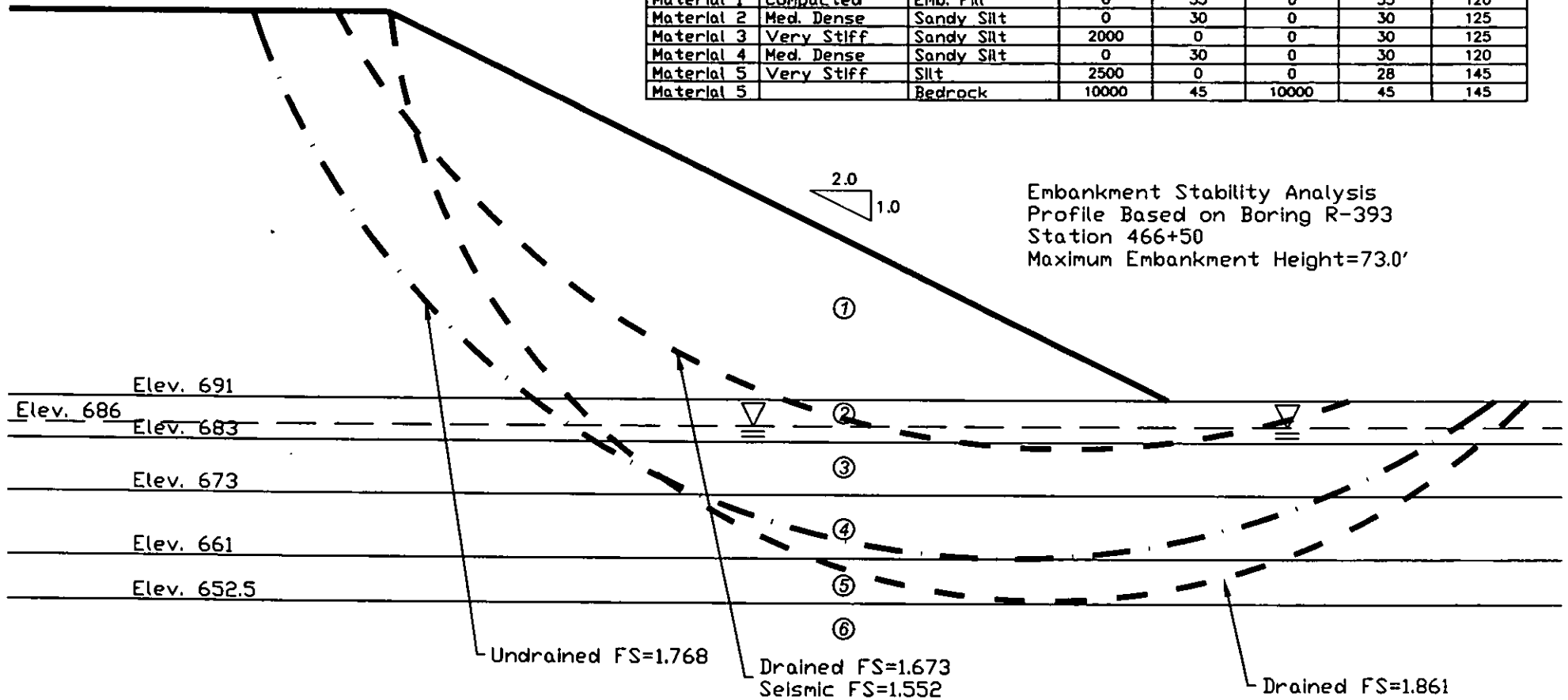
Figure:



APPENDIX C

Stability Analyses
Settlement Calculations

Material	Consistency	Soil Type	Undrained		Drained		γ (pcf)
			C (psf)	ϕ (deg)	C' (psf)	ϕ' (deg)	
Material 1	Compacted	Emb. Fill	0	35	0	35	120
Material 2	Med. Dense	Sandy Silt	0	30	0	30	125
Material 3	Very Stiff	Sandy Silt	2000	0	0	30	125
Material 4	Med. Dense	Sandy Silt	0	30	0	30	120
Material 5	Very Stiff	Silt	2500	0	0	28	145
Material 5		Bedrock	10000	45	10000	45	145



Embankment Global Stability
Station 466+50 Mainline Embankment
Profile Based upon boring R-393

MSE STABILITY ANALYSIS

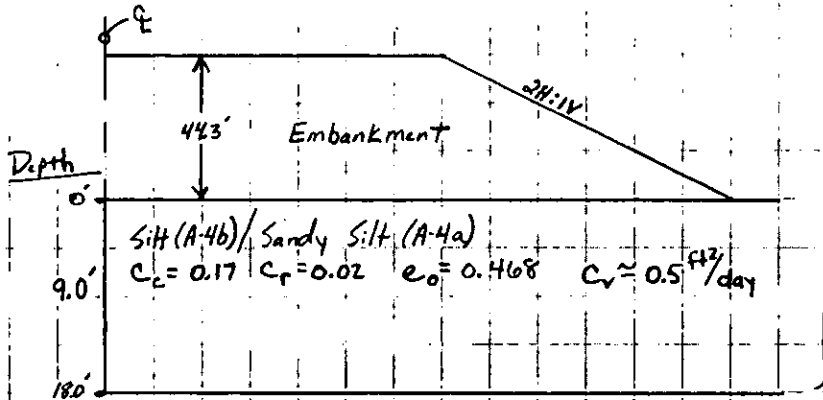
SCI-823-0.00

PROJECT NO. 0121-3070.03

CALC. SJR

DATE 11/25/06

* Most critical soil profile assumed at boring R-379.
At station 441+00, the maximum embankment height is 443'



Consolidation Parameters Estimated from
FHWA NHI-97-021.

Check if overconsolidated. $\frac{w-PL}{LL-PL} = \frac{17-22}{6} = -0.8 < 0.7$ ✓ Assume Overconsolidated.

$C_c = \frac{w}{100} = \frac{17}{100} = 0.17$ $C_r = \frac{w}{1000} = \frac{17}{1000} = 0.02$ $e_o = \frac{C_s \cdot w}{100} = \frac{2.75(17)}{100} = 0.468$

From attached calculation sheet:

$\bar{\sigma}_c = 2.1''$ - Consolidation of foundation soils

5.3'' - Estimated settlement of embankment fill. (10%).

Time-rate of Consolidation

* Assume Single drainage

$C_v = 0.5 \text{ ft}^2/\text{day}$

For $U = 80\%$ (80% Consolidation)

$\rightarrow T_v = 0.57$

$H_v = 18.0'$

$t_{80} = \frac{T_v \cdot H_v^2}{C_v}$

$t_{80} = \frac{(0.57)(18.0')^2}{0.5 \text{ ft}^2/\text{day}}$

$t_{80} = 369 \text{ days}$



SUBJECT

Client TranSystems Corp. / ODOT D-9

JOB NUMBER

0121-3070.03

Project SCI-823-6.81 Portsmouth Bypass

SHEET NO.

2 OF 6

Item Settlement Sta. 441+00, H=44.3'

COMP. BY

SJR DATE 11/25/06

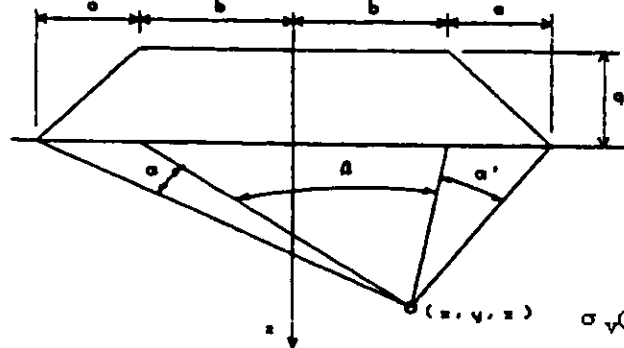
Profile Based Upon Boring R-379

CHECKED BY

gwt DATE 11-27-06

SETTLEMENT ANALYSIS - EMBANKMENT

Embankment Informaiton:



Groundwater Table: D= 18.0 ft
 Embankment Height: H= 44.3 ft
 Fill Unit Weight: $\gamma_{emb} = 120$ pcf $q = 5,316$ psf
 Width of Slope: a = 88.6
 Top half-width of Emb: b = 55
 Distance from CL: x = 0
 Output Range: z = 0 to 18 ft

*See Data output Attached

$$\sigma_v(z) := \left(\frac{q}{\pi a}\right) (a \cdot (\alpha(z) + \beta(z) + \alpha'(z)) + b \cdot (\alpha(z) + \alpha'(z)) + x \cdot (\alpha(z) - \alpha'(z)))$$

$$\beta(z) := \text{atan}\left[\frac{(b-x)}{z}\right] + \text{atan}\left[\frac{(b+x)}{z}\right]$$

$$\alpha'(z) := \text{atan}\left[\frac{(a+b-x)}{z}\right] - \text{atan}\left[\frac{(b-x)}{z}\right]$$

$$\alpha(z) := \text{atan}\left[\frac{(a+b+x)}{z}\right] - \text{atan}\left[\frac{(b+x)}{z}\right]$$

Reference: US Army Corps of Engineers EM 1110-1-1904 "Settlement Analysis", Table C-1

Soil Properties:

Settlement is calculated at mid-point of layer

Cohesionless

No.	Bot. of Laye	Soil Type	γ_{soil} (pcf)	σ'_c (psf)	σ'_o (psf)	$\Delta\sigma z$ (psf)	σ'_f (psf)	Cohesive Soils			
								C'	C_r	C_c	e_o
1	9.0 ft	Silt	120	5,856	540	5,316	5,856	0.0	0.02	0.17	0.468
2	8.0 ft	Sandy Silt	120	6,928	1,620	5,308	6,928	0.0	0.02	0.17	0.468
3	0.0		0	0							
4	0.0		0	0							
5	0.0		0	0							
6	0.0		0	0							
7	0.0		0	0							
8	0.0		0	0							
9	0.0		0	0							
10	0.0		0	0							

Reference: Geotechnical Engineering Principles and Practices; Coduto, 1999

Overconsolidated Soils - Case I ($\sigma'_o < \sigma'_c$) Eqn:11.24

$$(\delta_c)_{ult} = \sum \frac{C_r}{1+e_o} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Overconsolidated Soils - Case II ($\sigma'_o < \sigma'_c < \sigma'_f$) Eqn:11.25

$$(\delta_c)_{ult} = \sum \left[\frac{C_r}{1+e_o} H \log\left(\frac{\sigma'_c}{\sigma'_o}\right) + \frac{C_c}{1+e_o} H \log\left(\frac{\sigma'_f}{\sigma'_c}\right) \right]$$

Normally Consolidated Soils ($\sigma'_o = \sigma'_c$) Eqn: 11.23

$$(\delta_c)_{ult} = \sum \frac{C_c}{1+e_o} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Reference: FHWA NHI-00-045

Cohesionless Soils ($\sigma'_o = \sigma'_c$)

$$(\delta_c)_{ult} = \sum \frac{1}{C'} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

No. Settlement:

Total Settlement

1 0.108 ft

2 0.066 ft

3

4

5

6

7

8

9

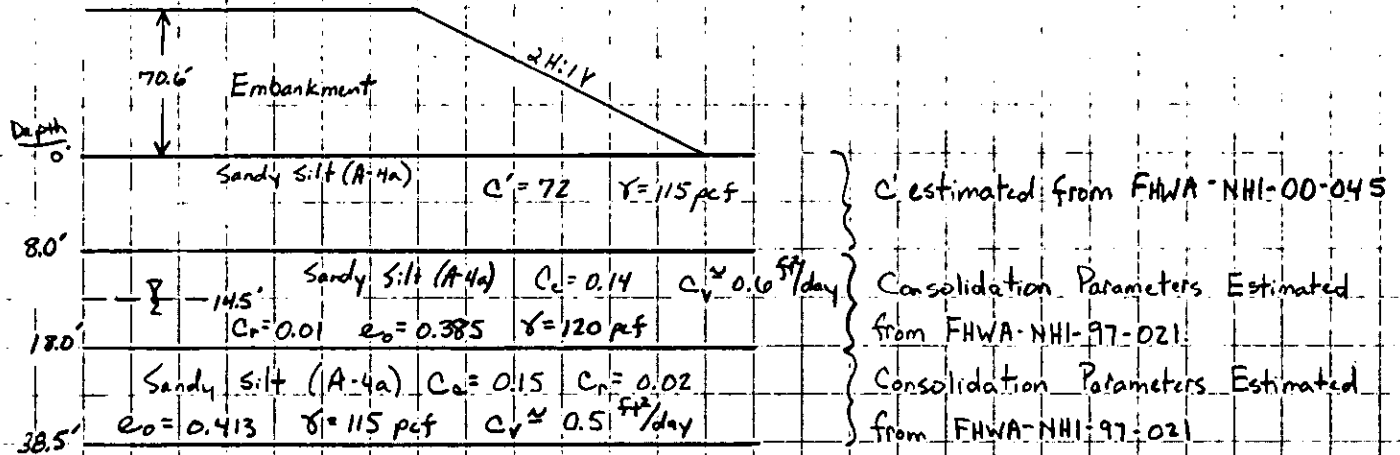
10

0.174 ft

2.1 in

* Most critical soil profile assumed at boring R-393.

At station 465+00, the maximum embankment height is approximately 70.6'.



Sample Calculations: Layer 2

Check if soil is overconsolidated.

$$\frac{w-PL}{LL-PL} = \frac{14-19}{24-19} = -1.0 < 0.7 \quad \text{Assume Overconsolidated}$$

$$C_c = \frac{w}{100} = \frac{14}{100} = 0.14 \quad C_r = \frac{w}{1000} = \frac{14}{1000} = 0.014 \quad e_0 = \frac{L_s \cdot w}{100} = \frac{2.75(14)}{100} = 0.385$$

From attached calculation sheet:

$$\sum e_c = 4.7''$$

1.7" Occurs instantly in granular layers.

3.0" Consolidation of foundation soils.

8.5" Estimated Settlement of embankment fill (1.0%)

Time-rate of Consolidation

$$t_{80} = \frac{T_v \cdot H_v^2}{C_v}$$

* Assume Single Drainage

$$C_v \approx 0.5 \text{ ft}^2/\text{day} \text{ (more conservative)}$$

$$t_{80} = \frac{(0.57)(30.5')^2}{0.5 \text{ ft}^2/\text{day}}$$

For $U=80\%$ (80% Consolidation)

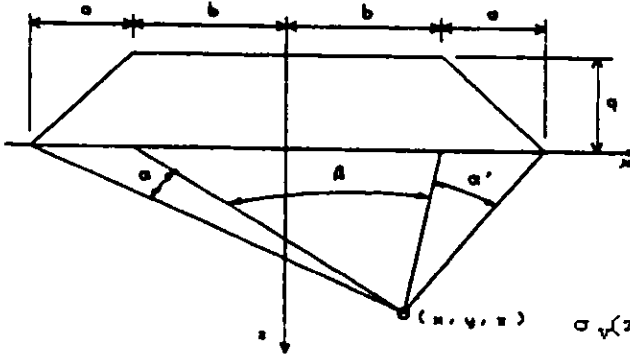
$$\rightarrow T_v = 0.57$$

$$t_{80} = 1060 \text{ days} = 2.9 \text{ years}$$

$$H_v = 38.5 - 8 = 30.5'$$

SETTLEMENT ANALYSIS - EMBANKMENT

Embankment Informaiton:



Groundwater Table: D= 14.5 ft
 Embankment Height: H= 70.6 ft
 Fill Unit Weight: $\gamma_{emb} = 120$ pcf $q = 8,472$ psf
 Width of Slope: a = 141.2
 Top half-width of Emb: b = 55
 Distance from CL: x = 0
 Output Range: z = 0 to 39 ft

*See Data output Attached

$$\sigma_v(z) := \left(\frac{q}{\pi a}\right) (a(\alpha(z) + \beta(z) + \alpha'(z)) + b(\alpha(z) + \alpha'(z)) + x(\alpha(z) - \alpha'(z)))$$

$$\beta(z) := \text{atan}\left[\frac{(b-x)}{z}\right] + \text{atan}\left[\frac{(b+x)}{z}\right] \quad \alpha'(z) := \text{atan}\left[\frac{(a+b-x)}{z}\right] - \text{atan}\left[\frac{(b-x)}{z}\right] \quad \alpha(z) := \text{atan}\left[\frac{(a+b+x)}{z}\right] - \text{atan}\left[\frac{(b+x)}{z}\right]$$

Reference: US Army Corps of Engineers EM 1110-1-1904 "Settlement Analysis", Table C-1

Soil Properties:

Settlement is calculated at mid-point of layer

Cohesionless

No.	Bot. of Laye	Soil Type	γ_{soil} (pcf)	σ'_c (psf)	σ'_o (psf)	$\Delta\sigma_z$ (psf)	σ'_f (psf)	Soils			
								C'	C _r	C _c	e _o
1	8.0 ft	Sandy Silt	115	8,932	460	8,472	8,932	72.0	0.00	0.00	0.000
2	18.0 ft	Sandy Silt	120	9,984	1,520	8,464	9,984	0.0	0.01	0.14	0.385
3	28.0 ft	Sandy Silt	115	10,596	2,165	8,431	10,596	0.0	0.02	0.15	0.413
4	38.5 ft	Sandy Silt	115	11,061	2,704	8,358	11,061	0.0	0.02	0.15	0.413
5	0.0		0	0							
6	0.0		0	0							
7	0.0		0	0							
8	0.0		0	0							
9	0.0		0	0							
10	0.0		0	0							

Reference: Geotechnical Engineering Principles and Practices; Coduto, 1999

Overconsolidated Soils - Case I ($\sigma'_o < \sigma'_c$) Eqn:11.24

$$(\delta_c)_{ult} = \sum \frac{C_r}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Overconsolidated Soils - Case II ($\sigma'_o < \sigma'_c < \sigma'_f$) Eqn:11.25

$$(\delta_c)_{ult} = \sum \left[\frac{C_r}{1+e_0} H \log\left(\frac{\sigma'_c}{\sigma'_o}\right) + \frac{C_c}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_c}\right) \right]$$

Normally Consolidated Soils ($\sigma'_o = \sigma'_c$) Eqn: 11.23

$$(\delta_c)_{ult} = \sum \frac{C_c}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Reference: FHWA NHI-00-045

Cohesionless Soils ($\sigma'_o = \sigma'_c$)

$$(\delta_c)_{ult} = \sum \frac{1}{C'} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

No. Settlement:

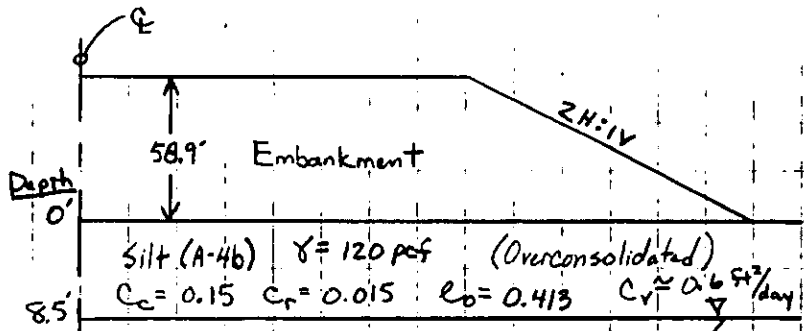
Total Settlement

1	0.143 ft
2	0.059 ft
3	0.098 ft
4	0.091 ft
5	
6	
7	
8	
9	
10	

0.391 ft

4.7 in

* Most critical soil profile assumed at boring B-10.
At station 487+00, the maximum embankment height is approximately 58.9'



Consolidation Parameters Estimated from FHWA N.H.L-97-021.

Sample Calculations

Check is soil overconsolidated $\frac{w - PL}{LL - PL} = \frac{15 - 19}{24 - 19} = -0.8 < 0.7$ ✓ Assum overconsolidated

$$C_c = \frac{w}{100} = \frac{15}{100} = 0.15 \quad C_r = \frac{w}{1000} = \frac{15}{1000} = 0.015 \quad e_o = \frac{G_s \cdot w}{100} = \frac{2.75(15)}{100} = 0.413$$

From attached calculation sheet:

$$\bar{\sigma}_c = 1.7''$$

7.1" Estimated Settlement of embankment fill (1.0%)

Time rate of consolidation

* Assume Single Drainage

$$C_v \approx 0.6 \text{ ft}^2/\text{day}$$

For $U = 80\%$ (80% Consolidation)

$$\rightarrow T_v = 0.57$$

$$H_v = 8.5'$$

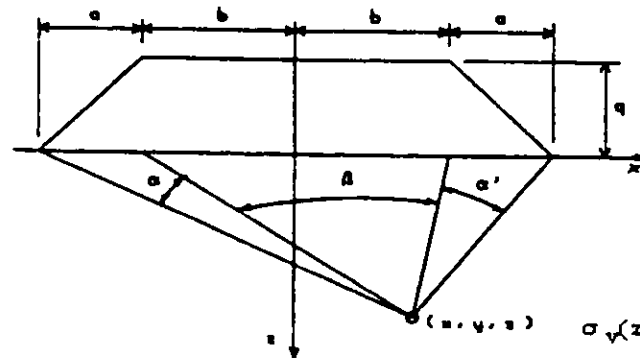
$$t_{80} = \frac{T_v \cdot H_v^2}{C_v}$$

$$t_{80} = \frac{(0.57)(8.5')^2}{0.6 \text{ ft}^2/\text{day}}$$

$$t_{80} = 68 \text{ days.}$$

SETTLEMENT ANALYSIS - EMBANKMENT

Embankment Informaiton:



Groundwater Table: D= 8.5 ft
 Embankment Height: H= 58.9 ft
 Fill Unit Weight: $\gamma_{emb} = 120$ pcf $q = 7,068$ psf
 Width of Slope: a = 117.8
 Top half-width of Emb: b = 55
 Distance from CL: x = 0
 Output Range: z = 0 to 9 ft

*See Data output Attached

$$\sigma_v(z) := \left(\frac{q}{\pi a}\right) (a \cdot (\alpha(z) + \beta(z) + \alpha'(z)) + b \cdot (\alpha(z) + \alpha'(z)) + x \cdot (\alpha(z) - \alpha'(z)))$$

$$\beta(z) := \text{atan}\left[\frac{(b-x)}{z}\right] + \text{atan}\left[\frac{(b+x)}{z}\right]$$

$$\alpha'(z) := \text{atan}\left[\frac{(a+b-x)}{z}\right] - \text{atan}\left[\frac{(b-x)}{z}\right]$$

$$\alpha(z) := \text{atan}\left[\frac{(a+b+x)}{z}\right] - \text{atan}\left[\frac{(b+x)}{z}\right]$$

Reference: US Army Corps of Engineers EM 1110-1-1904 "Settlement Analysis", Table C-1

Soil Properties:

Settlement is calculated at mid-point of layer

Cohesionless

No.	Bot. of Laye	Soil Type	γ_{soil} (pcf)	σ'_c (psf)	σ'_o (psf)	$\Delta\sigma_z$ (psf)	σ'_f (psf)	Soils			
								C'	C_r	C_c	e_o
1	8.5 ft	Silt	120	7,578	510	7,068	7,578	0.0	0.02	0.15	0.413
2	0.0		0	0							
3	0.0		0	0							
4	0.0		0	0							
5	0.0		0	0							
6	0.0		0	0							
7	0.0		0	0							
8	0.0		0	0							
9	0.0		0	0							
10	0.0		0	0							

Reference: Geotechnical Engineering Principles and Practices: Coduto, 1999

Overconsolidated Soils - Case I ($\sigma'_o < \sigma'_c$) Eqn:11.24

$$(\delta_c)_{ult} = \sum \frac{C_r}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Overconsolidated Soils - Case II ($\sigma'_o < \sigma'_c < \sigma'_f$) Eqn:11.25

$$(\delta_c)_{ult} = \sum \left[\frac{C_r}{1+e_0} H \log\left(\frac{\sigma'_c}{\sigma'_o}\right) + \frac{C_c}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_c}\right) \right]$$

Normally Consolidated Soils ($\sigma'_o = \sigma'_c$) Eqn: 11.23

$$(\delta_c)_{ult} = \sum \frac{C_c}{1+e_0} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

Reference: FHWA NHI-00-045

Cohesionless Soils ($\sigma'_o = \sigma'_c$)

$$(\delta_c)_{ult} = \sum \frac{1}{C'} H \log\left(\frac{\sigma'_f}{\sigma'_o}\right)$$

No. Settlement:

Total Settlement

1 0.141 ft

0.141 ft

2

3

4

5

1.7 in

6

7

8

9

10



APPENDIX D

ODOT Geotechnical Design Checklist

III.B. Embankments Checklist

C-R-S: SCI-823-6.81	PID: 19415	Reviewer: Steven Riedy	Date: 11-29-06
---------------------	------------	------------------------	----------------

Settlement

- Y N X 1 If soil conditions and project requirements warrant, have settlement issues been addressed?
If not applicable (X), go to Question 14
- Y N X 2 Have consolidation properties of the foundation soils been determined?
Check methods used:
 laboratory consolidation tests
 empirical correlations with moisture content and Atterberg values
 other
- Y N X 3 Have calculations been performed to estimate the total expected embankment settlement and the time of consolidation?
Check method used:
 EMBANK or equivalent software
 hand calculations
- Y N X 4 If differing foundation soil and/or loading conditions occur throughout the embankment area, have sufficient analyses been completed to evaluate consolidation at locations representative of the most critical conditions?
- Y N X 5 Have the total settlement and the time of consolidation analyses indicated acceptable values at all locations for the scope of the embankment work?
- Y N X 6 If total settlement or time of consolidation is unacceptable, have the stations and lateral extent of the problem areas been defined?
- Y N X 7 Has a method been chosen as a solution to the settlement issues?
Check methods used:
 waiting periods with monitoring
 drainage blanket and wick drains
 surcharge (preloading)
 removal and replacement of weak soil
 lowering proposed grade / change alignment
 lightweight fill
 other

List Other items:

III.B. Embankments Checklist

Y	N	<input checked="" type="radio"/>	8	Based on accepted design practices, and where applicable, adhering to published guidelines and design recommendations from FHWA, have calculations been performed to evaluate the effectiveness of the chosen solution(s)?		
Y	N	<input checked="" type="radio"/>	9	Has an economic analysis been performed to evaluate the cost benefits of the recommended solution compared to others?		
Y	N	<input checked="" type="radio"/>	10	Have all necessary notes, specifications, and details for the chosen solution been determined?		
Y	N	<input checked="" type="radio"/>	11	Have the need, locations, type, plan notes, and reading schedule for settlement platforms been determined?		
Y	N	<input checked="" type="radio"/>	12	Have the effects of the predicted settlement and the chosen solution been determined and accounted for on the construction schedule?		
Y	<input checked="" type="radio"/>	N	X	13	Has the effect of any foundation soil consolidation (including differential settlement) been evaluated with regard to adjacent structures (e.g., bridges, buildings, culverts, utilities) which will also undergo settlement and be subject to stresses induced by the consolidation of the surrounding soil?	The effect of the consolidation of foundational soils on the structures are evaluated in the respective MSE/Structure reports.

Notes :

Stage 1:

III.B. Embankments Checklist

Stability			
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X 14	<p>If soil conditions and project requirements warrant, have stability issues been addressed?</p> <p>If not applicable (X), go to Question 27</p>	
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X 15	<p>Has the total (short term) and effective (long term) shear strength of the foundation soils been determined?</p> <p>Check method used:</p> <p><input type="checkbox"/> laboratory shear tests</p> <p><input checked="" type="checkbox"/> estimation from SPT or field tests</p>	
Y	<input checked="" type="radio"/> N <input type="radio"/> X 16	<p>Have the OGE's recommended values of shear strength for proposed embankment fill material (total: c = 2000 psf, phi = 0; effective: c = 300 psf, phi = 28) been used in the stability analyses?</p>	<p>16.) Due to the large amount of rock to be excavated from adjacent cut sections, it is assumed that excavated rock will be used as fill to construct the embankments. The values selected are as follows; Total and Effective: cohesion = 0, friction angle = 35 degrees. Please refer to section 5.4.1 in the report for more information.</p>
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X 17	<p>Have calculations been performed to determine the F.S. for stability?</p> <p>Check method used:</p> <p><input checked="" type="checkbox"/> STABL, XSTABL, or equivalent software</p> <p><input type="checkbox"/> hand calculations</p>	
	18	<p>Have the following F.S. been met or exceeded, as determined by the calculations, for the given stability conditions:</p>	
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X	a 1.30 for short term condition	
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X	b 1.30 for long term condition	
Y	<input type="radio"/> N <input checked="" type="radio"/> X	c 1.10 for rapid drawdown, flood condition	
Y	<input type="radio"/> N <input checked="" type="radio"/> X	d 1.50 for embankment supporting bridge abutments (not on deep foundations)	
<input checked="" type="radio"/> Y	<input type="radio"/> N <input type="radio"/> X 19	<p>When differing soil or loading conditions occur throughout the embankment area, have sufficient analyses been completed to evaluate the stability at locations representative of the most critical conditions?</p>	
Y	<input type="radio"/> N <input checked="" type="radio"/> X 20	<p>If the F.S. was not met or exceeded, have the stations and lateral extent of the problem areas been defined?</p>	
Y	<input type="radio"/> N <input checked="" type="radio"/> X 21	<p>Has a method been chosen as a solution to the stability issues?</p> <p>Check the method(s) used:</p> <p><input type="checkbox"/> flattening slopes</p> <p><input type="checkbox"/> counter berm</p> <p><input type="checkbox"/> lightweight embankment</p>	

III.B. Embankments Checklist

- reinforced soil slope
- soil nailing
- drainage blanket and wick drains
- removal of soft soil, adding shear key
- reduced grade / change alignment
- stage construction
- controlled rate of fill placement
- drilled shaft slope stabilization
- other

List Other items:

- Y N 22 Based on accepted design practices, and where applicable, adhering to published guidelines and design recommendations from FHWA, have calculations been performed to evaluate the effectiveness of the chosen solution(s)?
- Y N 23 Has an economic analysis been performed to evaluate the cost benefits of the recommended solution compared to others?
- Y N 24 Have all necessary notes, specifications, and details for the chosen solution been determined?
- Y N 25 Have the need, location, type, plan notes, and reading schedule for piezometers and inclinometers been determined?
- Y N 26 If piezometers will be used, has the critical pressure value been determined and the appropriate information included in the plans?
- Y N 27 Have the effects of the stability solution been determined and accounted for on the construction schedule?
- Y N 28 Has the effect of the stability solution been evaluated with regard to structures (e.g., bridges, buildings, culverts, utilities) which may be subject to unusual stresses or require special construction considerations?

Notes:

Stage 1:

III.B. Embankments Checklist

Sidehill Fills	
<input checked="" type="radio"/> Y	N X 29 If soil conditions and project requirements warrant, have sidehill fill issues been addressed? If not applicable (X), go to Question 34
<input checked="" type="radio"/> Y	N X 30 In accordance with <u>Geotechnical Bulletin 2: Special Benching and Sidehill Embankment Fills (GB 2)</u> , have sidehill fills been evaluated to determine if special benching or shear keys are needed?
	31 In accordance with GB 2, if special benching or shear keys are required, has
<input checked="" type="radio"/> Y	N X a Plan Note G110 from L&D3 been included in the General Notes?
Y	N <input checked="" type="radio"/> X b quantities for both excavation and embankment been calculated for the benched areas and added to the plan General Quantities?
<input checked="" type="radio"/> Y	N X c the special benching or shear keys been indicated on the appropriate cross sections?
<input checked="" type="radio"/> Y	N X 32 Have water bearing zones been identified and their impact addressed?
<input checked="" type="radio"/> Y	N X 33 Have subsurface drainage controls been adequately addressed?

31 a & c.) Information, Plan Notes, and Cross Section have been provided to TranSystems Corporation.

Notes:

Stage 1:

III.B. Embankments Checklist

Special	
Y N <input checked="" type="radio"/>	34 Have all of the environmental factors, including wetlands, stream mitigation, and landfills, been considered and incorporated prior to design and analysis of embankment settlement and stability, including EPA or other government agencies' involvement, mitigation, or special design or construction considerations?
	35 If an embankment is to be placed through standing water or over weak, wet soils (with or without a fabric separator), the fill should be placed by the method of end dumping to a given height above the standing water or until compaction is achievable over the soft soil. If end dumping is to be specified,
<input checked="" type="radio"/> N X	a has the material type for the fill to be end dumped been specified?
Y <input checked="" type="radio"/> X	b has the need for a fabric separator or filter layer been determined?
<input checked="" type="radio"/> N X	c has the height of fill to be end dumped been determined?
Y <input checked="" type="radio"/> X	d have all notes and specifications for end dumping been developed?

Notes:

Stage 1: