

Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00						
LOG OF: Boring R-48				Location: Sta. 41+599.99, 81.96m Right of US 33 Centerline				Date Drilled: 6/27/00						
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m	
			Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SW	% Clay	PL	LL
						Water seepage at: none Water level at completion: dry (prior to core) 1.07 m (after coring, including core water)						Moisture Content - % Natural		
												X ————— X		
0	286.14					<b>DESCRIPTION</b>								
0.08	286.06					Topsoil - 0.076 m [3"]								
[0.3]	[938.5]					Stiff brown SILTY CLAY (A-6b); trace fine sand; damp.								
		4												
		4	203		1	239								
		6	181			[2.5]								
0.91	285.23					Hard brown and gray SILT AND CLAY (A-6a); trace fine to coarse sand; damp.								
[3.0]	[935.8]													
		4	279		2	383								
		8	111			[4.0]								
1.52	284.46					Hard red and brown SILT AND CLAY (A-6a); decomposed claystone; damp.								
[5.5]	[933.3]													
		9	356		3	431+								
		17	141			[4.5+]								
2.44	283.70					Very dense brown SANDY SILT (A-4a); decomposed siltstone; damp.								
[8.0]	[930.8]													
		50/10	102		4									
			[4]											
3.05														
[10]														
		30	229		5									
		50/08	191											
3.96	282.16					Hard red and brown SILT AND CLAY (A-6a); decomposed claystone; damp.								
[13.0]	[926.8]													
		50/13	127		6									
			[5]											
4.57						Soft brown and red CLAYSTONE; moderately weathered. @ 4.57 m - 4.86 m [15.0' - 16.0'], contains several shears with slickensides.								
[15]														
		Core	Rec											
		0.305	127			RQD								
		Core	Rec			RQD								
		1.524	1.499			43%								
		60"	59"											
6.10						@ 6.86 m - 7.01 m [22.5' - 23.0'], severely weathered.								
[20]														
		Core	Rec			RQD								
		1.524	1.372			73%								
		60"	54"											
7.62						@ 7.10 m [23.3'], slickensided shear.								







Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00											
LOG OF: Boring R-49				Location: Sta. 41+833.94, 76.00m Right of US 33 Centerline				Date Drilled: 6/30/00 - 7/6/00				STANDARD PENETRATION (N) Blows per 0.30 m							
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.	Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:	GRADATION						Moisture Content - %						
							% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL				
						Water seepage at: none Water level at completion: 0.91 m [3.0'] (after coring, including drill water)													
						<b>DESCRIPTION</b>													
0	286.16					Topsoil - 0.152 m [6"]													
0.15	286.01					Very soft red CLAYSTONE; severely weathered. @ 0.76 m [2.5'], becomes brown with rust staining. @ 1.22 m [4.0'], becomes gray. @ 1.83 m [6.0'], becomes soft.													
[0.5]	[938.4]	6																	
		11	.457		1							--							
		21	[18]																
		14	.432																
		17	[17]		2	96													
		25	.279			[1.0]													
		50	.13		3	--													
1.52							@ 1.83 m [6.0'], becomes soft.												
		50	.10	.102	4	--													
			[4]																
		50	.05	.051	5	--													
			[2]																
3.05	283.11						Soft grayish brown CLAYSTONE, calcareous, weathered, broken.												
[10]	[928.8]	Core 0.305	Rec 1.152	RQD 8%															
		Core 1.524	Rec 1.295	RQD 35%															
		m [60"]	m [51"]																
4.57							Medium hard gray SILTSTONE with limestone clasts, weathered, broken, with rust staining.												
4.69	281.47																		
[15.4]	[923.5]	Core 1.524	Rec 1.295	RQD 77%															
		m [60"]	m [51"]																
6.10							Soft gray CLAYSTONE: weathered to severely weathered.												
[20]		Core 1.524	Rec 1.092	RQD 35%															
		m [60"]	m [43"]																
7.44	278.72																		
7.62	[914.4]																		

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-49

Location: Sta. 41+833.94, 76.00m Right of US 33 Centerline Date Drilled: 6/30/00 - 7/6/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

X X X X

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

**WATER OBSERVATIONS:**  
 Water seepage at: none  
 Water level at completion: 0.91 m [3.0'] (after coring, including drill water)

GRADATION						
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	

DESCRIPTION						
7.62 [25]						Soft to medium hard gray CLAYSTONE, severely weathered.
		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 70%		
9.14 [30]						
		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 65%		
10.67 [35]						
		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 48%		
11.95 [39.2]	274.21 [899.6]					
12.19 [40]						Medium hard gray SILTSTONE, micaceous, severely weathered.
		Core 1.524 m [60"]	Rec 1.219 m [48"]	RQD 72%		
13.72 [45]						
		Core 1.524 m [60"]	Rec 1.118 m [44"]	RQD 30%		
15.24						



Client: Ohio Department of Transportation

Project: ATH-33-40.881

Job No. 9821-3200.00

LOG OF: Boring R-49

Location: Sta. 41+833.94, 76.00m Right of US 33 Centerline Date Drilled: 6/30/00 - 7/6/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

X ● X

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: 0.91 m [3.0'] (after coring, including drill water)	GRADATION						STANDARD PENETRATION (N)				Moisture Content - %			
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL	X	X			
22.86							<p><b>DESCRIPTION</b> Hard gray SANDSTONE, fine grained, slightly argillaceous, moderately weathered.</p> <p>Bottom of Boring - 23.16 m [76.0']</p>														
[75] 23.16 [76.0]	263.00 [862.9]																				
24.38																					
[80]																					
25.91																					
[85]																					
27.43																					
[90]																					
28.96																					
[95]																					
30.48																					









**Client:** Ohio Department of Transportation

**Project:** ATH-33-40.981

**Job No.** 9821-3200.00

**LOG OF:** Boring R-51

**Location:** Sta. 42+579.97, 41.95m Left of US 33 Centerline **Date Drilled:** 6/21/00

**STANDARD PENETRATION (N)**

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

X X

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	263.38					
0.15	263.23					
		4				
		4	406			
		8	[16]			
		14				
		17	432			
		31	[17]			
1.52						
1.68	261.70					
	[858.6]					
		50/10	102			
			[4]			
		50/10	102			
			[4]			
3.05	260.33					
	[854.1]					
		Core 1.219 m [48"]	Rec 1.219 m [48"]		RQD 100%	
4.57		Core 1.524 m [60"]	Rec 1.524 m [60"]		RQD 52%	
4.88	258.50					
	[848.1]					
		Core 1.524 m [60"]	Rec .406 m [16"]		RQD 20%	
6.10						
7.41	255.97					
	[839.8]					

**WATER OBSERVATIONS:**  
 Water seepage at: none  
 Water level at completion: none (prior to coring)  
 11.58 m [38.0'] (after coring, includes drilling water)

**GRADATION**

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

**DESCRIPTION**

Topsoil - 0.152 m [6"]

Medium dense brown SANDY SILT (A-4a), little clay; micaceous (decomposed sandstone); damp.

Soft brown SANDSTONE, micaceous, severely weathered.

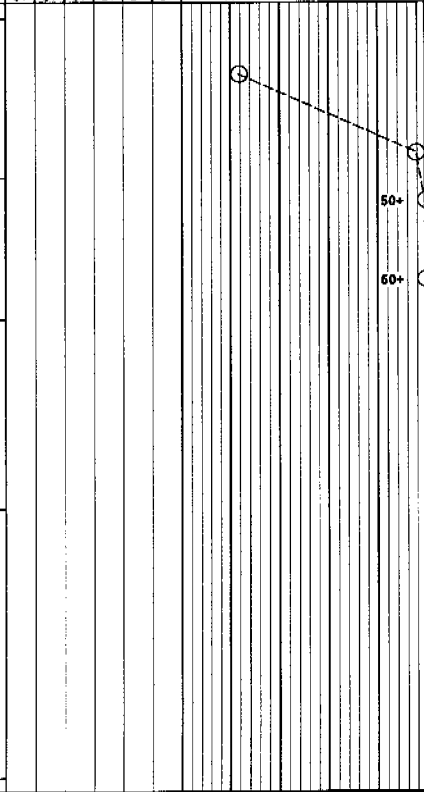
Medium hard brown SANDSTONE, fine-grained, micaceous; weathered.

@ 4.27 m [14.0'], soft, severely weathered.

Soft brown CLAYSTONE, weathered to severely weathered.

@ 5.64 m - 6.55 m [18.5' - 21.5'], medium hard to soft.

Very soft gray CLAYSTONE, severely weathered



Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-51

Location: Sta. 42+579.97, 41.95m Left of US 33 Centerline

Date Drilled: 6/21/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: none Water level at completion: none (prior to coring) 11.58 m [38.0'] (after coring, includes drilling water)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40 ○			
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL X ● X			
7.62 [25]	255.76 [839.1]			Core 1.524 m [60']	Rec 1.168 m [46']	RQD 68%		<b>DESCRIPTION</b>									
							Very soft gray CLAYSTONE, severely weathered. @ 8.29 m - 8.41 m [27.2' - 27.6'], broken zone.										
8.84 [29.0]	254.54 [835.1]			Core 1.524 m [60']	Rec 1.524 m [60']	RQD 72%	Very soft brown SANDSTONE, micaceous; severely weathered.										
9.11 [29.9]	254.27 [834.2]			Core 1.524 m [60']	Rec 1.524 m [60']		Very soft gray CLAYSTONE, micaceous; severely weathered.										
9.39 [30.8]	253.98 [833.3]			Core 1.524 m [60']	Rec 1.524 m [60']		Hard gray SANDSTONE, argillaceous, micaceous, pyritic, slightly calcareous, thin bedded, moderately weathered to weathered.										
10.67 [35]	253.76 [835.1]			Core 1.524 m [60']	Rec 1.473 m [58']	RQD 97%	@ 10.88 m - 10.97 m [35.7' - 36.0'], brown.										
11.00 [36.1]	252.38 [828.0]			Core 1.524 m [60']	Rec 1.473 m [58']		Hard gray SANDSTONE, fine to coarse grained, calcareous moderately weathered.										
11.52 [37.8]	251.86 [826.3]			Core 1.524 m [60']	Rec 1.473 m [58']		Hard gray LIMESTONE, slightly weathered.										
12.19 [40]	251.86 [826.3]			Core 1.219 m [48']	Rec 1.118 m [44']	RQD 88%	@ 12.18 m [40.0'], arenaceous and argillaceous.										
12.62 [41.4]	250.76 [822.7]			Core 1.219 m [48']	Rec 1.118 m [44']		Hard brown SANDSTONE, coarse grained, slightly weathered to weathered.										
13.11 [43.0]	250.27 [821.1]						Bottom of Boring - 13.11 m [43.0']										
13.72 [45]																	
15.24																	





Client: Ohio Department of Transportation

Project: ATH-33-40.981

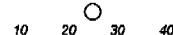
Job No. 9821-3200.00

LOG OF: Boring R-52

Location: Sta. 42+659.82, 124.13 m Rt. of US 33 Centerline Date Drilled: 7/5/00

STANDARD PENETRATION (N)

Blows per 0.30 m



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (prior to coring) 1.83 m [6.0'] (including core water)	GRADATION					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

**DESCRIPTION**

15.24	240.16					
[50]	[787.9]	Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 78%		
16.76						
[55]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 92%		
18.29						
[60]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 92%		
19.81						
[65]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 93%		
20.79	234.61					
[68.2]	[769.7]					
21.34						
[70.3]	[767.6]	Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 0%		
22.86	232.54					

Medium hard brown and gray SANDSTONE, fine grained micaceous, moderately weathered.

@ 15.70 m [51.5'], becomes gray.  
@ 15.88 m - 16.00 m [52.1' - 52.5'], very argillaceous, fractured with clay coating.

@ 16.79 m [55.1'], occasional dark gray laminae present.

@ 19.42 m [63.7'], carbonaceous laminae.

@ 20.07 m - 20.27 m [65.8' - 66.5'], medium grained.

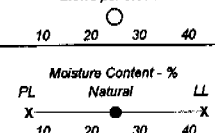
Medium hard gray SILTSTONE; contains sandstone laminae, weathered.

Soft black CARBONACEOUS SHALE with occasional coal layer, weathered.

Bottom of Boring - 22.86 m [75.0']



Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00													
LOG OF: Boring R-53				Location: Sta. 43+090.00, on Centerline of US 33 Centerline				Date Drilled: 6/20/00													
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m		Moisture Content - %					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	LL	Natural	LL				
0	256.44																				
0.09	256.35						DESCRIPTION														
[0.3]	[841.0]						Topsoil - 0.102 m [4"]														
0.91	255.53																				
[3.0]	[838.4]																				
1.52																					
[5]																					
2.44	254.00																				
[8.0]	[833.3]																				
3.05																					
[10]																					
3.78	252.66																				
[12.4]	[828.9]																				
4.57																					
[15]																					
6.10																					
[20]																					
7.62																					



% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
2	5	—	8	44	41

60+  
60+



Client: Ohio Department of Transportation

Project: ATH-33-40.981

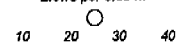
Job No. 9821-3200.00

**LOG OF: Boring R-55**

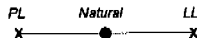
Location: Sta. 43+904.98, 0.20 m Right of US 33 Centerline Date Drilled: 6/19/00

STANDARD PENETRATION (N)

Blows per 0.30 m



Moisture Content - %



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m	Moisture Content - % PL Natural LL
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay		
0	242.31														
0.09	242.22														
[0.3]	[794.7]														
				3											
				4	279	1									
				4	[111]										
				5											
				16	457	2									
1.52				16	[181]										
1.68	240.63														
[5.5]	[789.5]														
				23											
				35	457	3									
				41	[181]										
2.44	239.87														
[8.0]	[787.0]														
				50/13	127	4									
					[5]										
3.05															
[10]															
				50/13	127	5									
3.47	238.84														
[11.4]	[783.6]				[5]										
4.57															
[15]															
6.10															
[20]															
7.62															

**DESCRIPTION**

Topsoil - 0.102 m [4"]

Hard reddish brown CLAY (A-7-6), trace fine to coarse sand, trace gravel; damp.

@ 1.37 m - 1.52 m [4.5' - 5.0'], slightly organic.

Hard brown SANDY SILT (A-4a), some clay; micaceous (decomposed siltstone); damp.

Soft brown SANDSTONE, argillaceous; severely weathered.

Bottom of Boring - 3.47 m [11.4']

LL=55%

76

50+

50+



















Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-61

Location: Sta. 45+039.80, 28.65m Lt. of US 33 Centerline

Date Drilled: 05/30-31/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (KN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER  
OBSERVATIONS:  
  
Water seepage at: none  
Water level at completion: 1.68 m [5.5'] (after coring, including  
drill water)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Blows per 0.30 m  
○  
10 20 30 40

Moisture Content - %  
PL Natural LL  
X ● X  
10 20 30 40

7.62 240.76

**DESCRIPTION**

[25]	[789.9]	Core 1.524 m [60"]	Rec .762 m [30"]	RQD 0%	Soft red, gray, and brown CLAYSTONE, micaceous, severely weathered, arenaceous, contains limestone, siltstone, and sandstone clasts.  @ 7.32 m - 10.36 m [24.0' - 34.0'], gas encountered.
9.14 [30]		Core 1.524 m [60"]	Rec .483 m [19"]	RQD 0%	

Bottom of Boring - 10.36 m [34.0']

10.36 238.02

[34.0] [780.9]

10.67 [35]

12.19 [40]

13.72 [45]

15.24











Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00							
LOG OF:		Boring R-64		Location: Sta. 45+020.00, 73.51 m Rt. of US 33 Centerline				Date Drilled: 05/23/00							
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m	
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	LL
							Moisture Content - % Natural						X		
							X						X		
0	215.93						<b>DESCRIPTION</b>								
0.06	215.87						Topsoil - 0.051 m [2"]								
[0.2]	[708.2]						Stiff to very stiff brown SILT AND CLAY (A-6a); trace fine to coarse sand, trace gravel; moist. @ 0.73 m [2.4], thin organic layer.								
0.91	215.02		2	203	1	192	Soft gray CLAYSTONE, weathered.								
[3.0]	[705.4]		4	[8]		[2.0]									
1.52															
1.62	214.31														
[5.3]	[703.1]														
							Bottom of Boring - 1.62 m [5.3']								
3.05															
[10]															
4.57															
[15]															
6.10															
[20]															
7.62															













Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00															
LOG OF: Boring R-69			Location: Sta. 45+805.25, 27.86m Rt. of US 33 Centerline				Date Drilled: 5-11-00			STANDARD PENETRATION (N) Blows per 0.30 m													
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: none Water level at completion: none	GRADATION						Moisture Content - %									
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL							
0	213.89																						
0.05	213.84																						
[0.2]	[701.6]																						
		2																					
		3	.152	[6]																			
0.92	212.97																						
[3]	[698.7]																						
1.19	212.70																						
[3.9]	[697.8]	20	.406																				
		24	.191																				
		8																					
		12	.229																				
		14	.091																				
2.71	211.18																						
[8.9]	[692.8]	12	.305																				
3.05		50/.08	.121																				
[10]																							
3.44	210.45	50/.08	.076																				
[11.3]	[690.4]		.031																				
4.57																							
[15]																							
6.10																							
[20]																							
7.62																							

**DESCRIPTION**

Topsoil - 0.051 m [2"]

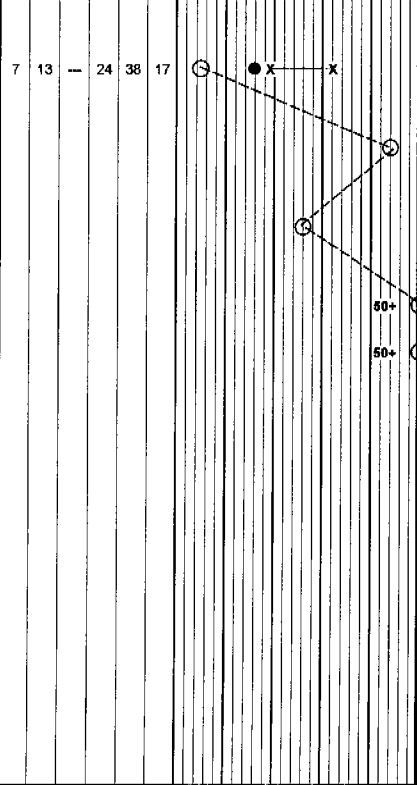
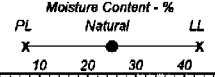
Stiff to very stiff brown SILT AND CLAY (A-6a); little fine to coarse sand, trace gravel, contains roots; moist.

Very stiff gray and dark gray SILT AND CLAY (A-6a); trace to little fine to coarse sand; damp to moist.  
@ 1.13 m - 1.16 m [3.7' - 3.8'], organic layer.

Medium dense gray COARSE AND FINE SAND (A-3a); trace to little silty clay, laminated; damp.

Soft brown and gray CLAYSTONE, severely weathered to highly weathered.  
@ 3.20 m [10.5'], becomes gray.

Bottom of Boring - 3.44 m [11.3']





Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00												
LOG OF: Boring R-70				Location: Sta. 45+860.02, 0.03 m Lt. of US 33 Centerline				Date Drilled: 5-11-00				STANDARD PENETRATION (N) Blows per 0.30 m								
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: none Water level at completion: none (before coring) 0.15 m [0.5'] (after coring)	GRADATION						Moisture Content - %						
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL				
0	233.15																			
0.10	233.05																			
[0.3]	[764.6]																			
		4																		
		3	.203 [8]		1															
0.92	232.23																			
[3]	[761.9]																			
		50/10	.102 [4]		2															
1.52																				
[5]																				
		50/08	.076 [3]		3															
		50/08	.076 [3]		4															
3.05	230.10																			
[10]	[754.9]																			
		Core 1.219 m [48"]	Rec 1.143 m [45"]		RQD 94%															
4.57																				
[15]																				
		Core 1.524 m [60"]	Rec 1.524 m [60"]		RQD 92%															
5.24	227.91																			
[17.2]	[747.7]																			
6.10																				
[20]																				
		Core 0.762 m [30"]	Rec .762 m [30"]		RQD 77%															
6.49	226.66																			
[21.3]	[743.6]																			
		Core 0.762 m [30"]	Rec .686 m [27"]		RQD 33%															
7.62																				

**DESCRIPTION**

Topsoil - 0.102 m [4"]

Loose dark brown GRAVEL WITH SAND AND SILT (A-2-4); damp.

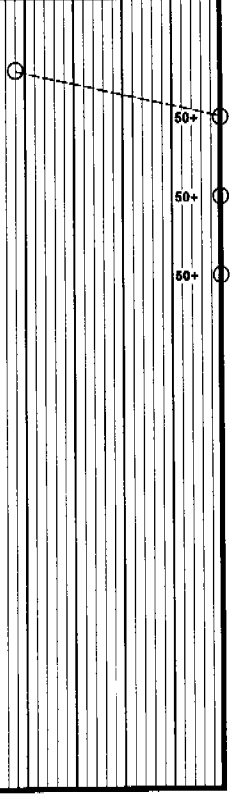
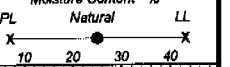
Soft brown and gray SANDSTONE, slightly micaceous, severely weathered to weathered.

Medium hard to hard brown and gray SANDSTONE, micaceous, weathered, slightly broken.

@ 4.63 m [15.2'], weathered to slightly weathered, poorly cemented.  
@ 4.79 m [15.7'], clay seam with rust stains.

Medium hard gray SANDSTONE; moderately weathered, contains coal blossums, poorly cemented.

Soft black COAL and carbonaceous SHALE, very broken, weathered.  
@ 6.55 m - 7.32 m [21.5' - 24.0'], gas encountered.  
@ 6.86 m [22.5'], becomes dark gray.  
@ 7.35 m [24.1'], becomes interbedded with gray and dark brown SANDSTONE.





Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00							
LOG OF: Boring R-71				Location: Sta. 45+899.81, 28.09 m Lt. of US 33 Centerline				Date Drilled: 5-9-00 & 5-10-00							
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: 0.67 m [2.2] Water level at completion: none (before coring) 4.88 m [16.0] (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m	
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LI	
0	247.88						<b>DESCRIPTION</b>								
0.13	247.73						Topsoil - 0.127 m [5"]								
[0.4]	[812.8]			1	.178 [7]	1	Stiff brown SILT AND CLAY (A-6a), trace to little fine to coarse sand; contains roots; damp.								
0.92	246.94			3	[7]										
[3]	[810.2]			50/08	.076 [3]	2	Soft brown SANDSTONE, slightly micaceous, severely weathered to weathered.								
1.52	246.34														
[5]	[808.2]			Core 1.219 m [48"]	Rec 1.118 m [44"]	RQD 92%	Medium hard to hard, light gray SANDSTONE, calcareous, micaceous, weathered.								
2.50	245.36						@ 2.01 m - 2.13 m [6.6' - 7.0'], ligh angle fracture, severely weathered.								
[8.2]	[805.0]			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%	Hard, gray LIMESTONE, slightly micaceous, weathered to moderately weathered.								
3.05							@ 3.64 m [11.9'], low angle fracture.								
[10]															
4.27	243.59			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%	Medium hard to hard, brown SANDSTONE, micaceous, moderately weathered, slightly broken.								
[14.0]	[799.2]														
4.57															
[15]															
5.03	242.83						Hard, light gray LIMESTONE, slightly micaceous, moderately weathered to weathered, contains brown and gray micaceous sandstone layers.								
[16.5]	[796.7]														
6.10				Core 1.524 m [60"]	Rec 1.067 m [42"]	RQD 70%	@ 6.04 m [19.8'], becomes broken.								
[20]							@ 6.55 m [21.5'], fractured with calcite staining.								
7.25	240.61														
[23.8]	[789.4]						Medium hard to hard, brown and gray SANDSTONE, micaceous, slightly broken, contains limestone clasts.								
7.62															

Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00												
LOG OF: Boring R-71				Location: Sta. 45+899.81, 28.09 m Lt. of US 33 Centerline				Date Drilled: 5-9-00 & 5-10-00				STANDARD PENETRATION (N) Blows per 0.30 m ○ 10 20 30 40								
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Core [m] [ft]	Rec [m] [ft]	Sample No.	Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 0.67 m [2.2'] Water level at completion: none (before coring) 4.88 m [16.0'] (after coring)	GRADATION						Moisture Content - % Natural						
								% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL X	LL X					
7.62	240.24						<b>DESCRIPTION</b>													
[25] 7.62	[788.2] 240.24		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%		Medium hard to hard, brown and gray SANDSTONE, micaceous, slightly broken, contains limestone clasts.													
[30] 9.14			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%		<p>@ 7.32 m - 11.89 m [24.0' - 39.0'], gas encountered.</p> <p>@ 10.30 m [33.8'], near vertical healed fracture.</p> <p>@ 10.67 m - 10.73 m [35.0' - 35.2'], low angle clay coated fracture.</p>													
[35] 10.67			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%															
[40] 12.19			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 90%		<p>@ 12.01 m - 12.16 m [39.4' - 39.9'], very broken.</p> <p>@ 12.16 m - 12.34 m [39.9' - 40.5'], argillaceous.</p> <p>@ 12.34 m [40.5'], dark brown laminae becomes evident.</p>													
[45] 13.72			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%		<p>@ 13.99 m [45.9'], very thin clay seam.</p> <p>@ 14.08 m [46.2'], very thin clay seam.</p> <p>@ 14.11 m [46.3'], very thin clay seam.</p> <p>@ 14.17 m [46.5'], becomes very broken, highly fractured.</p>													
15.24							@ 14.94 m - 16.46 m [49.0' - 54.0'], gas encountered.													











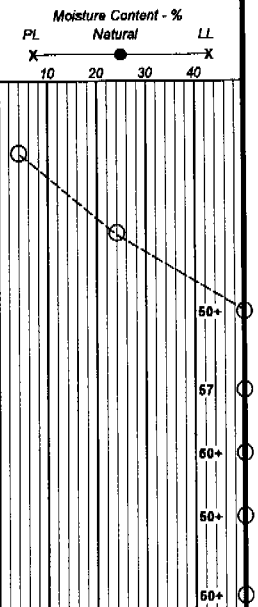








Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00												
LOG OF: Boring R-74				Location: Sta. 46+375.29, 50.07 m Rt. of US 33 Centerline				Date Drilled: 5/3/00 - 5/5/00												
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m [ft]	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kNm <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: none Water level at completion: none (prior to core) 3.66 m [12] (including drill water)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40						
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL		X ——— X				
0	257.80																			
<b>DESCRIPTION</b>																				
0.40	257.40	1					Topsoil - 0.406 m [16"]													
[1.3]	[844.5]	2	229			1	14.4	Stiff reddish brown SILTY CLAY (A-6b); damp to moist. @ 0.91 m [3.0'], becomes very stiff (decomposed claystone).												
		6																		
1.52	256.12	10	356			2	—													
[5.5]	[840.3]	14	[14]																	
		21						Hard brown SILTY CLAY (A-6b), trace fine to coarse sand; (decomposed claystone); damp.												
		45	381			3	—													
		50/10	[15]																	
		20																		
3.05		22	457			4	—	@ 3.20 m [10.5'], little fine to coarse sand.												
[10]		35	[18]																	
		20	278																	
		50/13	[11]			5	—													
3.96	253.84																			
[13.0]	[832.8]																			
		50/13	.102			6		Soft dark brown SANDSTONE, slightly micaceous, severely weathered to weathered.												
			[4]																	
4.57	253.08																			
[15.5]	[830.3]																			
		50/05	.051			7		Soft gray LIMESTONE, severely weathered.												
			[2]																	
5.33	252.47																			
[17.5]	[828.3]	Core	Rec			RQD		Hard gray LIMESTONE, slightly weathered. @ 5.49 m [18.0'], water loss.												
		0.762	533			80%														
		m	m																	
		[30"]	[21"]																	
6.10	251.70																			
[20]	[825.8]	Core	Rec			RQD		Hard brown SANDSTONE, micaceous, slightly weathered to weathered.												
		1.524	1,524			100%														
		m	m																	
		[60"]	[60"]																	
7.56	250.24																			
[24.8]	[821.0]							Hard gray SANDSTONE, micaceous, moderately weathered to weathered.												







Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-74

Location: Sta. 46+375.29, 50.07 m Rt. of US 33 Centerline

Date Drilled: 5/3/00 - 5/5/00

STANDARD PENETRATION (N)

Blows per 0.30 m

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER  
OBSERVATIONS:

Water seepage at: none  
Water level at completion: none (prior to core)  
3.66 m [12] (including drill water)

## GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
-------------	-----------	-----------	-----------	--------	--------

Moisture Content - %  
Natural

PL  
X ————— X  
LL

22.86 234.94

## DESCRIPTION

[75]		Core 1.524 m [50']	Rec 1.499 m [50']	RQD 98%		
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Medium hard brown and gray SANDSTONE, micaceous.

24.38

[80]		Core 1.524 m [50']	Rec 1.499 m [50']	RQD 98%		
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Ⓢ 24.41 m [80.1], clay seam.

Ⓢ 24.75 m [81.2], small Claystone clasts.

25.91 231.89

[85]	[760.8]					
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Bottom of Boring - 25.91 m [85.0']

27.43

[90]						
------	--	--	--	--	--	--

28.96

[95]						
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30.48





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

**LOG OF: Boring R-75**

Location: Sta. 48+521.02, 28.90 m Rt. of US 33 Centerline

Date Drilled: 5/1/00 & 5/2/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro-meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (prior to core) none (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40						
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL				
7.62 [25]	239.49 [785.7]						<b>DESCRIPTION</b>													
8.23 [27.0]	238.88 [783.7]	Core 3.048 m [120"]	Rec 3.048 m [120"]					Hard brown and gray SANDSTONE, micaceous, moderately weathered.												
8.84 [29.0]	238.27 [781.7]							Hard light gray SANDSTONE, calcareous, moderately weathered, micaceous.												
9.14 [30]								Medium hard light gray SANDSTONE, micaceous, weathered.												
9.78 [32.1]	237.33 [778.7]							Hard light gray LIMESTONE, micaceous, moderately weathered. @ 10.49 m [34.4'], brown SANDSTONE bedding becomes evident. @ 10.36 m - 11.89 m [34.0' - 39.0'], gas encountered.												
10.67 [35]		Core 1.524 m [60"]	Rec 1.524 m [60"]			RQD 97%														
11.89 [39.0]	235.22 [771.7]						Bottom of Boring - 11.89 m [39.0']													
12.19 [40]																				
13.72 [45]																				
15.24																				

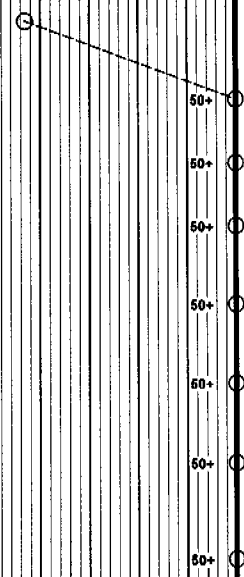
Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00					
LOG OF: Boring R-76				Location: Sta. 46+520.00, 53.00 m Rt. of US 33 Centerline				Date Drilled: 5/3/00					
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 1.83 m [6.0'] Water level at completion: none (prior to core) 0.91 m [3.0'] (including drill water)	GRADATION					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
							STANDARD PENETRATION (N) Blows per 0.30 m ○ 10 20 30 40						
							Moisture Content - % PL Natural X —●— X LL 10 20 30 40						
<b>DESCRIPTION</b>													
0	249.41						Topsoil - 0.381 m [15"]						
0.38 [1.3]	249.03 [817.0]	2	5 9	457 [18]		1	Medium dense reddish brown COARSE AND FINE SAND (A-3a), little to some silt and clay; damp. @ 0.91 m [3.0'], dark brown, micaceous.						
1.52 [5]		3	6 8	457 [18]		2							
		8	10 13	457 [18]		3	@ 2.44 m [8.0'], very dense, brown with trace to little silt and clay.						
3.05 [10]		18	27 38	457 [18]		4	@ 3.20 m [10.5'], brown and dark brown.						
		21	35 43	457 [18]		5							
3.96 [13.0]	245.45 [805.3]	4	4	.229 [9]		6	287 [3.0] Very stiff brown SILTY CLAY (A-6b), trace fine to coarse sand, trace gravel; damp.						
4.57 [15.5]	244.69 [802.8]	29	27 39	457 [18]		7	431+ [4.5+] Hard brown SILT AND CLAY (A-6a), trace to little fine to coarse sand; micaceous; damp.						
5.49 [18.0]	243.92 [800.3]	50/13	.127 [5]			8	Soft brown SHALE, severely weathered.						
6.10 [20]	243.01 [797.3]	50/05	.051 [2]			9	Soft brown SANDSTONE, severely weathered.						
7.32 [24.0]	242.09 [784.3]	50/08	.076 [3]			10	Medium hard brown and gray SANDSTONE, moderately weathered						
7.62													







Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00														
LOG OF: Boring R-79				Location: Sta. 46+860.00, 52.00 m Rt. of US 33 Centerline				Date Drilled: 4/24/00														
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: 5.64 m [18.5] Water level at completion: 1.37 m [4.5] (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m ○								
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL X ● X								
0	250.56						<b>DESCRIPTION</b> Topsoil - 0.102 m [4"] Stiff brown SILT AND CLAY (A-6a), little to some fine to coarse sand; contains roots; organic; damp to moist. Hard brown SILT (A-4b), trace fine to coarse sand; damp. Soft brown SANDSTONE, micaceous, argillaceous, severely weathered.															
0.09	250.47																					
[0.3]	[821.8]																					
0.43	250.13	2																				
[1.4]	[820.6]	3	381	1A																		
		4	[15]	1B																		
0.92	249.64																					
[3]	[819.0]	15																				
		26	381	2																		
		50/08	[15]																			
1.52		44	229	3																		
[5]		50/08	[9]																			
		50/13	.127	4																		
			[5]																			
3.05		50/13	.127	5																		
[10]			[5]																			
		50/13	.127	6																		
			[5]																			
4.57		50/08	.076	7																		
[15]			[3]																			
		37	254	8																		
		50/13	[10]																			
6.10	244.46																					
[20]	[802.0]	Core	Rec																			
		1.219	0.940																			
		m [48"]	m [37"]																			
6.86	243.70																					
[22.5]	[799.5]																					
7.62																						













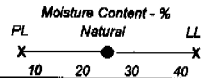
Client: Ohio Department of Transportation Project: ATH-33-40.981 Job No. 9821-3200.00

LOG OF: Boring R-81 Location: Sta. 47+054.056, 0.08 m Rt. of US 33 Centerline Date Drilled: 4/18 & 4/19/00

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (before coring) 0.61 m [2.0'] (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m			
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	10	20	30	40

**DESCRIPTION**

7.62 [25]	239.92 [787.1]						Soft to medium hard gray SILTSTONE, micaceous, broken, contains calcareous banding, weathered.  @ 8.35 m [27.4'], high angle fracture.  @ 9.60 m - 9.72 m [31.5' - 31.9'], highly weathered, broken.  Hard, gray SANDSTONE, micaceous, pyritic, slightly calcareous with argillaceous interbeds, weathered to slightly weathered.													
		Core 1.524 m [60"]	Rec 1.118 m [44"]			RQD 10%														
9.14 [30]		Core 1.524 m [60"]	Rec 1.524 m [60"]			RQD 17%														
10.03 [32.9]	237.51 [778.2]	Core 1.524 m [60"]	Rec 1.524 m [60"]			RQD 88%														
10.67 [35]																				
11.58 [38.0]	235.96 [774.2]																			
12.19 [40]																				
13.72 [45]																				
15.24							Bottom of Boring - 11.58 m [38.0']													







Client: Ohio Department of Transportation

Project: ATH-33-40.881

Job No. 9821-3200.00

LOG OF: Boring R-83

Location:

Sta. 47+179.92, 0.32 m Rt. of US 33 Centerline

Date Drilled: 4/20/00

STANDARD PENETRATION (N)

Blows per 0.30 m

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Spec (m)	Sample No.		Hand Penetrom- eter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	242.24					
0.37	241.87 [793.5]					
[1.2]			1		.229 [9]	1
0.92	241.32 [791.7]					
[3]			3			
			4		.356 [14]	2
1.52						
[5]			6			
			13		.356 [14]	3A
			23			3B
			10			
			13		.408 [16]	4
3.05						
[10]						
			13			
			19		.178 [7]	5
			21			
			4			
4.42	237.82		16		.330 [13]	6
4.57	[780.2]		50/13			
[15.0]						
			50/10		.102 [4]	7
			50/08		.076 [3]	8
6.10						
6.25	235.99					
[20.5]	[774.2]		50/05		.051 [2]	9
			50/10		.102 [4]	10
7.92						

WATER  
OBSERVATIONS:Water seepage at: none  
Water level at completion: none

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Moisture Content - %

## DESCRIPTION

Topsoil - 0.356 m [14"]

Very stiff brown and gray SILT AND CLAY (A-6a); slightly organic; contains roots; moist.

Very stiff brownish gray SILT AND CLAY (A-6a), trace fine to coarse sand; damp to moist.

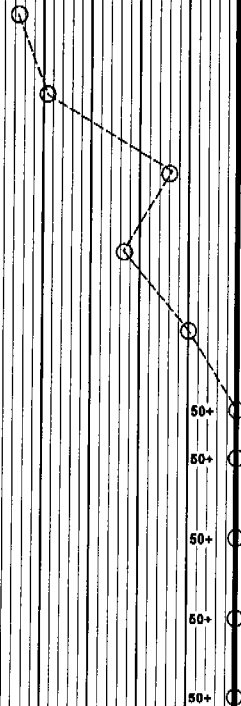
@ 1.68 m - 3.20 m [5.5' - 10.5'], becomes red and brown.

431+  
431+  
[4.5+]431+  
[4.5+]431+  
[4.5+]

Soft brownish gray SHALE, micaceous, severely weathered.

Soft gray SANDSTONE, micaceous, severely weathered.

@ 6.25 m - 7.01 m [20.5' - 23.0'], calcareous.







Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-84

Location: Sta. 47+200.08, 28.06 m Lt. of US 33 Centerline

Date Drilled: 4/19/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

X ————— X

10 20 30 40

WATER OBSERVATIONS:

Water seepage at: 11.68 m [38.5]  
Water level at completion: none

GRADATION

% Aggregate  
% C. Sand  
% M. Sand  
% F. Sand  
% Silt  
% Clay

DESCRIPTION

Topsoil - 0.203 m [8"]

Stiff to very stiff brown SILT AND CLAY (A-6a), trace to little fine to coarse sand, trace gravel; contains roots; damp.

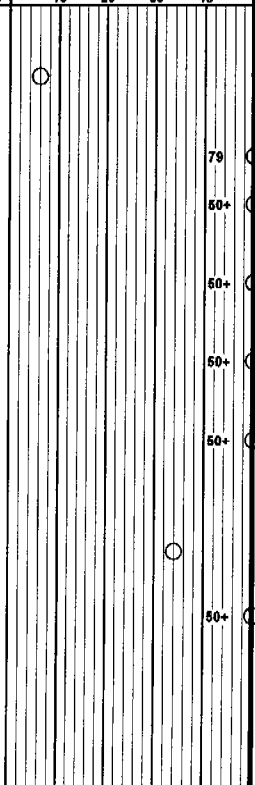
Very dense brown COARSE AND FINE SAND (A-3a), trace silty clay; damp.

Soft brown SANDSTONE, argelaceous, severely weathered, micaceous.

Very soft gray CLAYSTONE, severely weathered to weathered. @ 4.72 m - 5.49 m [15.5' - 18.0'], very soft.

Soft gray CLAYSTONE, weathered.

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m)	Sample No.		Hand Penetro-meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	248.42					
0.21	248.21					
[0.7]	[814.3]					
0.92	247.50	1 2 4	203 [8]		1	
[3]	[812.0]					
1.52	246.74	30 30	330 [13]		2	
1.88	246.74	49				
[5.5]	[809.5]					
		50/13	127 [5]		3	
		50/08	076 [3]		4	
3.05						
[10]						
		50/10	102 [4]		5	
		50/08	076 [3]		6	
4.57						
4.73	243.69					
[15.5]	[799.5]					
		13 15 19	406 [16]		7	
		39 50/13	279 [11]		8	
6.10	242.32					
[20]	[795.0]					
		Core 1.219 m [48"]	Rec 1.143 m [45"]	RQD 63%		
7.82						







Client: Ohio Department of Transportation					Project: ATH-33-40.981					Job No. 9821-3200.00								
LOG OF: Boring R-85					Location: Sta. 47+840.04, 65.12 m Lt. of US 33 Centerline					Date Drilled: 4/14/00				STANDARD PENETRATION (N) Blows per 0.30 m				
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kNm <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:		GRADATION						Moisture Content - %			
				Dive	Press		Water seepage at: 2.59 m [8.5']		% Aggregats	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Natural			
							Water level at completion: 5.64 m [18.5']								PL X ————— X Natural LL			
															10 20 30 40			
7.62	234.00						<b>DESCRIPTION</b>							10 20 30 40				
[25]	[767.7]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 83%		Medium hard gray SANDSTONE, fine grained micaceous; weathered.											
9.14							@ 9.14 m [30.0'], 0.05 m [0.15'] clay seam.											
[30]			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 82%		@ 9.69 m - 9.81 m [31.8' - 32.2'], clay seam.											
10.67							@ 10.67 m - 10.79 m [35.0' - 35.4'], fine to medium grained.											
[35]			Core 3.048 m [120"]	Rec 3.023 m [119"]	RQD 96%													
12.19							@ 12.77 m - 13.08 m [41.9' - 42.9'], fine to medium grained.											
[40.0]																		
13.53	228.09																	
[45.0]	[748.3]		Core 2.438 m [96"]	Rec 2.413 m [95"]	RQD 81%		Medium hard dark gray SILTSTONE, micaceous, weathered, with fine to medium grained sandstone.											
15.24																		

Client: Ohio Department of Transportation      Project: ATH-33-40.981      Job No. 9821-3200.00

LOG OF: Boring R-85      Location: Sta. 47+840.04, 65.12 m Lt. of US 33 Centerline      Date Drilled: 4/14/00      STANDARD PENETRATION (N)  
Blows per 0.30 m

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:	GRADATION					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

15.24 226.38  
15.39 [742.7]

**DESCRIPTION**

Medium hard dark gray SILTSTONE, micaceous, weathered, with fine to medium grained sandstone.

16.15 225.47  
[53.0] [739.7]

Medium hard brown SANDSTONE, fine grained, micaceous, slightly arillaceous, weathered.

Bottom of Boring - 16.15 m [53.0']

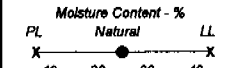
16.76  
[55]

18.29  
[60]

19.81  
[65]

21.34  
[70]

22.88



Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-86

Location: Sta. 47+939.99, 45.01 m Lt. of US 33 Centerline

Date Drilled: 4/17/00

STANDARD PENETRATION (N)

Blows per 0.30 m

○

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	237.21					
0.40 [1.3]	236.81 [776.9]	1	.356 [1.4]			
0.91 [3.0]	236.30 [775.3]	3				
1.52 [5.0]	235.53 [772.7]	5	.279 [1.1]			
2.44 [8.0]	234.77 [770.2]	23	.330 [1.3]			
3.05 [10]	234.18 [768.2]	24 50/10	.254 [1.0]			
4.11 [14.2]	233.10 [764.0]					
4.33 [14.2]	232.88 [764.0]	Core 2.997 m [118']	Rec 2.997 m [118']	RQD 87%		
6.10 [20]	231.11 [758.2]					
7.62						

WATER  
OBSERVATIONS:

Water seepage at: 3.35 m [11']  
Water level at completion: 3.96 m [13.0'] (before coring)  
1.83 m [6.0'] (after coring)

## GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Moisture Content - %

Natural

PL ——— LL

X ——— X

10 20 30 40

## DESCRIPTION

Topsoil - 0.406 m [16"]

Stiff brown SILT AND CLAY (A-6a), trace fine to coarse sand; contains roots; damp to moist.

Very stiff to hard reddish brown SILTY CLAY (A-6b), trace fine to coarse sand; damp.

Hard reddish brown and light brown SILT AND CLAY (A-6a), trace fine to coarse sand; damp.

Soft, light brown SILTSTONE, micaceous, argillaceous, severely weathered to decomposed.

Soft, light brown, SANDSTONE, fine to coarse grained, micaceous, severely weathered.

Medium hard, gray SANDSTONE, calcareous, micaceous, contains siltstone layers, weathered.

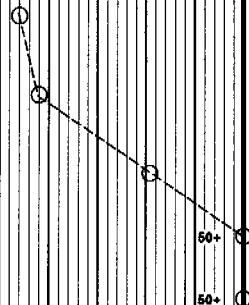
Hard, brown SANDSTONE, argillaceous, micaceous, weathered


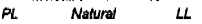
@ 4.36 m - 4.57 m [14.3' - 15.0'], highly broken with high angle fractures, severely weathered.

@ 5.18 m [17.0'], becomes brown and gray.

Hard, gray SILTSTONE, micaceous, weathered, contains occasional claystone layers.

@ 6.83 m [22.4'], high angle fracture.



Client: Ohio Department of Transportation					Project: ATH-33-40.981					Job No. 9821-3200.00														
LOG OF: Boring R-86					Location: Sta. 47+939.99, 45.01 m Lt. of US 33 Centerline					Date Drilled: 4/17/00														
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 3.35 m [11'] Water level at completion: 3.96 m [13.0'] (before coring) 1.83 m [6.0'] (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 										
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL 										
7.62	229.59																							
<b>DESCRIPTION</b>																								
[25]	[753.2]	Core 3.048 m [120']	Rec 3.048 m [120']	RQD 98%	Hard, gray SILTSTONE, micaceous, weathered, contains occasional claystone layers.																			
8.87	228.34				@ 8.84 m [29.0'], low angle rust stained fracture.																			
[29.1]	[749.1]				Hard, gray and brown SANDSTONE, slightly broken, weathered to severely weathered.																			
9.14					@ 8.86 m [29.4'], low angle rust stained fracture.																			
[30]					@ 9.39 m [30.8'], low angle rust stained fracture.																			
					@ 9.62 m [31.5'], low angle rust stained fracture.																			
					@ 9.08 m [29.8'], clay seam.																			
					@ 9.69 m - 9.88 m [31.8' - 32.4'], argillaceous with clay layers.																			
		Core 3.048 m [120']	Rec 3.048 m [120']	RQD 100%	@ 10.15 m [33.3'], becomes gray, weathered to slightly weathered.																			
10.67																								
[35]																								
					@ 11.31 m [37.1'], high angle fracture.																			
					@ 11.52 m [37.8'], shale clast.																			
12.19																								
[40]																								
					@ 12.44 m [40.8'], shale clasts.																			
13.20	224.01																							
[43.3]	[734.9]				Bottom of Boring - 13.20 m [43.3']																			
13.72																								
[45]																								
15.24																								



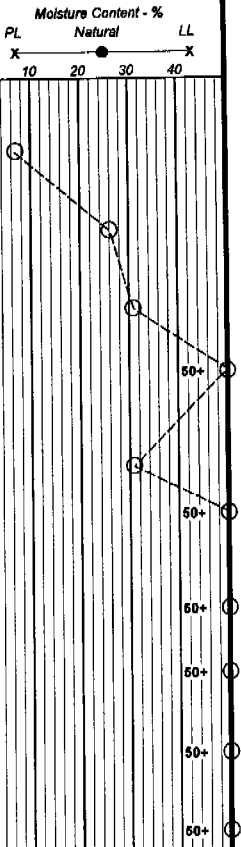






Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00												
LOG OF: Boring R-88				Location: Sta. 48+194.71, 34.22 m Rt. of US 33 Centerline				Date Drilled: 4/18/00												
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 1.83 m [6.0'] Water level at completion: none	GRADATION					STANDARD PENETRATION (N) Blows per 0.30 m							
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL X X X X						
7.62 [25]	206.58 [677.8]						<b>DESCRIPTION</b>  Very soft to soft gray and dark gray SHALE, severely weathered.  Bottom of Boring - 8.35 m [27.4']													
8.35 [27.4]	205.85 [675.4]					11														
9.14 [30]																				
10.67 [35]																				
12.19 [40]																				
13.72 [45]																				
15.24																				

Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00														
LOG OF: Boring R-89			Location: Sta. 48+229.28, 0.39 m Rt. of US 33 Centerline				Date Drilled: 4/13/00				STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40											
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	GRADATION						Moisture Content - %									
				Drive	Press		% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SW	% Clay	PL	Natural	LL							
0	223.82						<b>DESCRIPTION</b>															
0.12	223.70						Topsoil - 0.127 m [5"]															
0.4	733.9						Soft to medium stiff brown SILT AND CLAY (A-6a), some fine to coarse sand; contains rootlets; moist.															
0.91	222.91			1	356	1	96 [1.0]	Medium dense brown COARSE AND FINE SAND (A-3a), trace to little clay and silt; moist.														
3.0	731.3			6	141	1		@ 1.68 m [5.5'], becomes dense.														
1.52				11	.457	2		@ 2.44 m - 3.20 m [8.0' - 10.5'], very dense.														
5				13	.181	2		@ 3.20 m [10.5'], brown and gray.														
				5				Soft brown SANDSTONE, slightly micaceous, severely weathered, friable.														
				10	.457	3		@ 4.88 m [16.0'], brown and gray, argillaceous, and micaceous.														
				21	.181	3		@ 5.03 m [16.5'], contains coal blossoms.														
3.05				32	.229	4																
10				50/08	.191	4																
				11	.381	5																
				14	.151	5																
3.96	219.86			17		5																
13.0	721.3			50/13	.127	6																
4.57				37	.229	7A																
15				50/08	.191	7B																
				50/10	.102	8																
6.10				50/08	.076	9	@ 6.40 m - 7.62 m [21.0' - 25.0'], contains seepage.															
20																						
				50/08	.076	10																
7.62																						





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

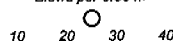
LOG OF: Boring R-91

Location: Sta. 48+379.99, 10.00m Lt. of US 33 Mainline

Date Drilled: 4/12/00-4/13/00

STANDARD PENETRATION (N)

Blows per 0.30 m



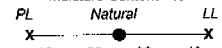
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
			Rec (m) [ft]	Drive Press	
0	233.43				
0.09	233.33				
[0.3]	[765.5]				
		1			
			8	.381 [15]	1
0.91	232.51				
[3.0]	[762.8]				
		12			
		29	.457 [18]		2
1.52					
[6.5]	[760.3]				
		31			
		44	.457 [18]		3
2.44	230.98				
[8.0]	[757.8]				
		9			
		12	.279 [11]		4
3.05					
[10]					
		17	.178 [7]		5
		50/0.3			
		21	.229 [9]		6
4.42	229.00				
[15.0]	[751.3]				
		4.57			
		50/0.08			
		Core 1.016 m [40"]	Rec 1.016 m [40"]		RQD 88%
		Core 1.524 m [60"]	Rec 1.524 m [60"]		RQD 100%
6.10					
[20]					
7.62					

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: none (before coring)  
 1.5 (after coring)

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Moisture Content - %



DESCRIPTION

Topsoil - 0.102 m [4"]

Very stiff brown SILT AND CLAY (A-6a), some to "and" fine to coarse sand; contains roots; damp.

Very dense brown COARSE AND FINE SAND (A-3a), little to some silty clay; contains roots; micaceous, damp.

Soft, brown, fine to coarse grained SANDSTONE, micaceous, severely weathered to decomposed.

Very soft, brown SILTSTONE, argillaceous, arenaceous, micaceous, severely weathered to decomposed.

@ 3.35 m [11.0'], becomes soft and non-arenaceous.

@ 4.11 m [13.5'], becomes non-argillaceous.

Hard, gray, SANDSTONE, argillaceous, fine to very fine grained, micaceous, weathered.

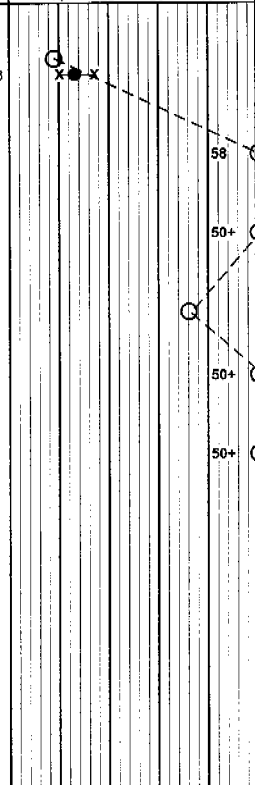
@ 5.39 m [17.7'], becomes brown.

@ 5.46 m - 5.58 m [17.9' - 18.3'], arenaceous shale layer.

@ 5.90 m - 6.07 m [19.4' - 19.9'], thin clay seam.

@ 5.97 m [19.6'], very thin incomplete shale laminae.

@ 6.25 m [20.5'], sandstone clasts.



Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-91

Location: Sta. 48+379.99, 10.00m Lt. of US 33 Mainline

Date Drilled: 4/12/00-4/13/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (before coring) 1.5 (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40			
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - % PL Natural LL X ● X			
7.62							<b>DESCRIPTION</b>										
[25]		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%		Hard, brown, SANDSTONE, argillaceous, fine to very fine grained, micaceous, weathered.											
9.14						@ 8.84 m - 8.93 m [29.0' - 29.3'], claystone, broken with slickensided shears.											
[30]						@ 8.93 m - 9.05 m [29.3' - 29.7], interbedded sandstone and claystone.											
						@ 9.60 m [31.5'], clay seam.											
10.67		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%		@ 11.09 m - 12.65 m [36.4' - 41.5'], friable.											
[35]																	
12.19						@ 13.17 m - 13.38 m [43.2' - 43.9'], argillaceous.											
[40]																	
13.72		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 98%		@ 14.75 m [48.4], becomes friable.											
[45]																	
15.24																	

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

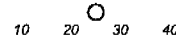
LOG OF: Boring R-91

Location: Sta. 48+379.99, 10.00m Lt. of US 33 Mainline

Date Drilled: 4/12/00-4/13/00

STANDARD PENETRATION (N)

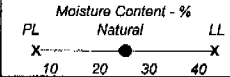
Blows per 0.30 m



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro-meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: none (before coring)  
 1.5 (after coring)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay



15.24

**DESCRIPTION**

[50]		Core 3,048 m [120"]	Rec 3,048 m [120"]	RQD 98%		
------	--	---------------------	--------------------	---------	--	--

Hard, brown, SANDSTONE, argillaceous, fine to very fine grained, micaceous, weathered.  
 @ 15.51 m - 15.67 m [50.9' - 51.4], claystone laver.  
 @ 15.82 m - 15.91 m [51.9' - 52.2], broken.

16.46 216.97

[54.0] [741.8]

16.76

[55]

18.29

[60]

19.81

[65]

21.34

[70]

22.86

Bottom of Boring - 16.46 m [54.0']



Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00								
LOG OF:		Boring R-92		Location: Sta. 48+480.00, 30.00m Lt. of US 33 Mainline				Date Drilled: 4/11/00-4/12/00				STANDARD PENETRATION (N) Blows per 0.30 m				
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	GRADATION						Moisture Content - %			
				Drive	Press		% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL	
0	231.37						WATER OBSERVATIONS: Water seepage at: 14.6' Water level at completion: 14.6' (before coring) 34' (after coring)						10 20 30 40			
0.12	231.25												<b>DESCRIPTION</b>			
[0.4]	[758.7]						<b>Topsoil - 0.127 m [5"]</b>									
		1					Medium dense brown COARSE AND FINE SAND (A-3a). some silty clay; contains organic plant material. micaceous; damp to moist.									
		2	.406			1										
		9	[16]													
		13														
1.52		18	.432			2	@ 1.07 m [3.5'], becomes dense and organic plant material disappears.									
[5]		26	[17]													
		24														
		47	.432			3	@ 1.83 m [6.01]. becomes very dense. trace silty clay.									
2.44	228.93	50/13	[17]													
[8.0]	[751.1]															
		33					Soft, brown SANDSTONE, fine to coarse grained, severely weathered to decomposed.									
		53	.406			4										
3.05		50/10	[16]													
[10]																
		31	.279			5	@ 3.35 m [11.0'], becomes micaceous and slightly argillaceous.									
		50/13	[11]													
		30	.203			6										
4.42	226.95	50/76	[8]													
4.57	[744.6]															
[15.0]		Core 2.743 m [108"]	Rec 2.210 m [87"]				Medium hard, brown SANDSTONE, slightly broken, argillaceous, weathered. @ 4.72 m - 4.82 m [15.5' - 15.8']. claystone seam.									
							RQD 78%									
6.10							@ 6.80 m [22.3']. clay seam.									
[20]																
							@ 7.01 m - 7.07 m [23.0' - 23.2']. clay coated fractures.									
7.62																

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-92

Location: Sta. 48+480.00, 30.00' Lt. of US 33 Mainline

Date Drilled: 4/11/00-4/12/00

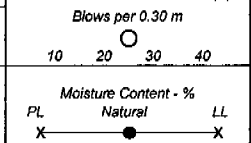
STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.30 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER  
OBSERVATIONS:

Water seepage at: none  
Water level at completion: 14.6' (before coring)  
34' (after coring)

GRADATION						
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	



DESCRIPTION						
7.62 [25]	223.75 [734.1]	Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%		Medium hard, brown SANDSTONE, slightly broken, argillaceous, moderately weathered. @ 8.08 m [26.5'], clay seam.  @ 8.41 m - 9.60 m [27.6' - 31.5'], friable.
9.14 [30]						
10.67 [35]		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%		
12.19 [40]						@ 12.07 m - 13.41 m [39.6' - 44.0'], severely weathered, broken.
13.72 [45]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%		@ 13.29 m - 13.41 m [43.6' - 44.0'], highly broken. @ 13.41 m [44.0'], gas encountered.
14.94 [49.0] 15.24	216.43 [710.1]					Bottom of Boring - 14.94 m [49.0']

Client: Ohio Department of Transportation						Project: ATH-33-40.981						Job No. 9821-3200.00							
LOG OF: Boring R-93						Location: Sta. 48+478.12, 29.38m Rt. of US 33 Mainline						Date Drilled: 4/19/00							
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (KN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL			
							DESCRIPTION							Moisture Content - %					
0	219.70						Topsoil - 0.457 m [18"]									X			X
0.46	219.24	WOH	.000				Medium dense brown and gray COARSE AND FINE SAND (A-3a). trace silty clay; moist.												
[1.5]	[719.3]	WOH 6	[0]																
1.07	218.83	8	.305				Soft, brown SANDSTONE, argillaceous, micaceous, severely weathered.												
[3.8]	[717.0]	50/08	.076		1	2													
1.52							Bottom of Boring - 1.16 m [3.8']												
							Note: Boring sampled by hand using light, cable-percussion drilling equipment.												
3.05																			
[10]																			
4.57																			
[15]																			
6.10																			
[20]																			
7.62																			

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

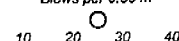
LOG OF: Boring R-94

Location: Sta. 48+619.91, on Centerline of US 33 Mainline

Date Drilled: 4/11/00

STANDARD PENETRATION (N)

Blows per 0.30 m



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m		Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
		Rec (m)	[ft]	Drive	Press	
0	221.12					
0.15	220.97					
0.5	[725.0]					
		1	.457			
		2	[18]			
		1	.457			
		2	[18]			
1.52						
[5]						
		2	.457			
		4	[18]			
		2	.457			
		4	[18]			
		3	.406			
		8	[16]			
3.05						
[10.5]	217.92					
	[715.0]					
		24	.279			
		50/13	[11]			
		31	.203			
		50/08	[8]			
4.57						
[15]						
		52	.203			
		50/08	[8]			
		20	.279			
		50/13	[11]			
6.10						
[20]						
		41	.254			
		50/10	[10]			
7.01	214.11					
[23.0]	[702.5]					
		28	.279			
		50/13	[11]			
7.62						

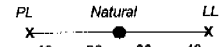
WATER OBSERVATIONS:

Water seepage at: 28.5'  
Water level at completion: 8.53 m [28.0']

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
0	18	—	45	21	16

Moisture Content - %



DESCRIPTION

Topsoil - 0.152 m [6"]

Very loose brown COARSE AND FINE SAND (A-3a). some to "and" silty clay; contains roots; moist.

@ 1.07 m [3.5'], becomes loose, brown and gray, contains "and" silty clay.

@ 1.83 m [6.0'], becomes medium dense, slightly micaceous, contains trace silty clay.

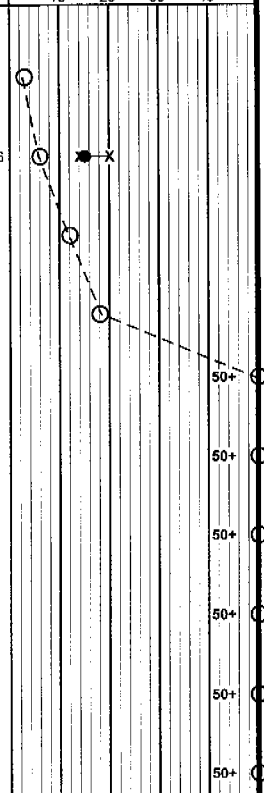
Soft gray SILTSTONE, micaceous, argillaceous, severely weathered to decomposed.

@ 4.88 m [16.0'], non-micaceous.

@ 5.64 m [18.5'], becomes argillaceous, micaceous.

@ 6.40 m - 6.86 m [21.0' - 22.5'], contains organic fossiliferous plant material and coal deposits.

Soft, black COAL



Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

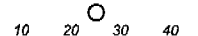
LOG OF: Boring R-94

Location: Sta. 48+619.91, on Centerline of US 33 Mainline

Date Drilled: 4/11/00

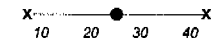
STANDARD PENETRATION (N)

Blows per 0.30 m



Molsture Content - %

PL Natural LL



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro-meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: 28.5' Water level at completion: 8.53 m [28.0']	GRADATION						STANDARD PENETRATION (N)						
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	10	20	30	40			
7.62 [25]	213.50 [700.5]						<b>DESCRIPTION</b>  Soft, gray SILTSTONE, argillaceous, severely weathered to decomposed.													
		50/08	.076 [3]	11																50+
8.81 [28.9]	212.31 [696.5]	50/13	.127 [5]	12			Bottom of Boring - 8.81 m [28.9']												50+	
9.14 [30]																				
10.67 [35]																				
12.19 [40]																				
13.72 [45]																				
15.24																				





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-95

Location: Sta. 48+780.84, 30.34m Lt. of US 33 Mainline

Date Drilled: 4/5/00-4/11/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

X-----●-----X

10 20 30 40

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: none (before coring)  
 6.40 m [21.0'] (after coring)

GRADATION

% Aggregate  
 % C. Sand  
 % M. Sand  
 % F. Sand  
 % Silt  
 % Clay

DESCRIPTION

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.	Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
15.24	216.00				
[50]	[708.7]	Core 1.524 m [60"]	Rec 1.422 m [56"]	RQD 93%	
16.76		Core 1.524 m [60"]	Rec 1.499 m [59"]	RQD 87%	
[55]					
17.92	213.32				
[58.8]	[699.9]	Core 1.524 m [60"]	Rec 1.348 m [53"]	RQD 22%	
18.29					
[60]					
18.62	212.62				
[61.1]	[697.6]				
19.05	212.19				
[62.5]	[696.2]				
19.48	211.76				
[63.9]	[694.8]	Core 1.524 m [60"]	Rec 3.005 m [12"]	RQD 0%	
19.81					
[65]					
21.34		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 88%	
[70]					
22.86					

Medium hard brown SANDSTONE, micaceous; poorly cemented; broken; weathered; contains argillous clasts: friable.  
 @ 15.64 m - 15.70 m [51.3' - 51.5'], arav.  
 @ 15.88 m - 15.97 m [52.1' - 52.4'], arav.  
 @ 16.28 m - 16.43 m [53.4' - 53.9'], arav.  
 @ 16.64 m - 16.79 m [54.6' - 55.1], arav with coal stringers.  
 @ 17.43 m - 17.89 m [57.2' - 58.7'], near vertical fracture, highly weathered.  
 @ 17.60 m - 17.72 m [57.8' - 58.2'], highly broken.  
 @ 17.83 m - 17.91 m [58.5' - 58.8'], arav.  
 Soft black COAL, weathered.  
 Soft arav CLAYSTONE, weathered to slightly weathered.  
 Medium hard to hard arav LIMESTONE, weathered.  
 @ 19.35 m [63.5'], broken.  
 Soft arav CLAYSTONE, slightly micaceous; contains siltstone clasts: slightly weathered.  
 @ 19.51 m - 21.40 m [64.0' - 70.2], highly broken.  
 @ 21.03 m - 22.56 m [69.0' - 74.0], encountered aas.  
 @ 21.79 m - 21.92 m [71.5' - 71.9], highly broken.  
 @ 22.31 m - 22.37 m [73.2' - 73.4], reddish.





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

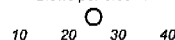
LOG OF: Boring R-97

Location: Sta. 49+001.73, 105.42m Rt. of US 33 Mainline

Date Drilled: 3/21/00-3/23/00

STANDARD PENETRATION (N)

Blows per 0.30 m



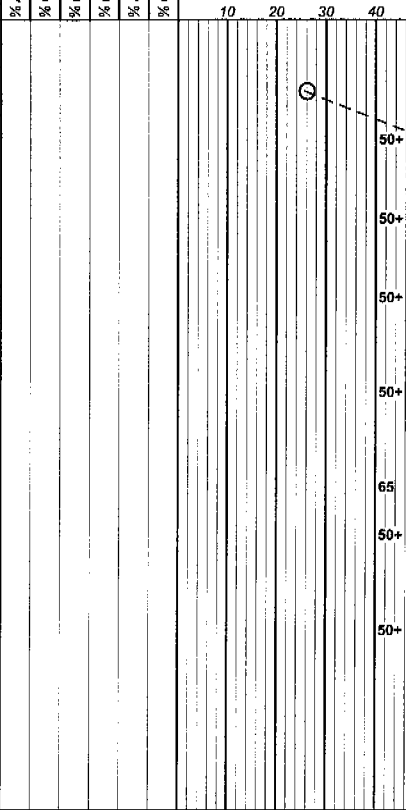
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro-meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	247.00					
0.37 [1.2]	246.63 [809.1]	4	8	.356 [14]		1
0.91 [3.0]	246.09 [807.4]					
1.52 [5]			50/13	.127 [5]		2
			50/10	.102 [4]		3
2.29 [7.5]	244.71 [802.8]					
			50/13	.127 [5]		4
3.05 [10]						
			17	.279 [11]		5
			50/13			
3.96 [13.0]	243.04 [797.4]					
			12	.330 [13]		6
4.57 [15.5]	242.28 [794.9]					
			50/08	.078 [3]		7
5.33 [17.5]	241.67 [792.9]					
			33	.254 [10]		8
5.94 [20.0]	241.06 [790.9]					
			Core 1.219 m [48']	Rec .813 m [32']		RQD 67%
7.62						

WATER OBSERVATIONS:  
 Water seepage at: 0.91 m [3.0']  
 Water level at completion: dry (before coring)  
 dry (after coring)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay



DESCRIPTION						
Topsoil - 0.356 m [14"]						
Medium dense brown COARSE AND FINE SAND (A-3a), some silty clay; damp.						
Soft brown, SANDSTONE, fine to coarse grained, micaceous, argillaceous; severely weathered to decomposed.						
@ 1.83 m [6.0'], becomes severely weathered.						
Soft brown SILTSTONE, micaceous; severely weathered to decomposed.						
Very soft dark brown CLAYSTONE, severely weathered to decomposed.						
Soft gray SILTSTONE, severely weathered.						
Soft dark gray SHALE, argillaceous, severely weathered.						
Soft to medium hard gray CLAYSTONE, slightly weathered to weathered, broken						
@ 7.32 m [24.0'], highly broken.						



Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

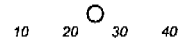
**LOG OF: Boring R-97**

Sta. 49+001.73, 105.42m Rt. of US 33 Mainline

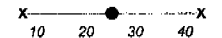
Date Drilled: 3/21/00-3/23/00

STANDARD PENETRATION (N)

Blows per 0.30 m



Moisture Content - %  
Natural



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.	
				Drive	Press
7.62	239.38				
9.14					
9.51	237.49				
10.36	236.64				
10.67					
10.82	236.18				
12.19					
13.72					
15.24					

WATER OBSERVATIONS:  
Water seepage at: 0.91 m [3.0']  
Water level at completion: dry (before coring)  
dry (after coring)

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

**DESCRIPTION**

Depth (m) [ft]	Elev. (m) [ft]	Core (m) [ft]	Rec (m) [ft]	RQD
7.62	239.38	Core 1.524 m [60"]	Rec 1.524 m [27"]	RQD 0%
9.14				
9.51	237.49	Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 75%
10.36	236.64			
10.67				
10.82	236.18	Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%
12.19				
13.72		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%
15.24				

Soft to medium hard gray CLAYSTONE: slightly weathered to weathered, broken

@ 8.84 m [29.0'] limestone.

Medium hard gray SILTSTONE, micaceous, weathered, contains limestone clasts.

@ 10.73 m [35.2'], thin clay seam.  
@ 10.76 m - 10.82 m [35.3' - 35.5'], limestone lense.

Medium hard brown SILTSTONE, arenaceous, micaceous, slightly calcareous, weathered.

Medium hard to hard brown SANDSTONE, micaceous, weathered.

@ 10.82 m - 12.07 m [35.5' - 39.6'], argillaceous, very thin bedded.  
@ 12.07 m [39.6'], becomes light brown.  
@ 12.07 m - 12.65 m [39.6' - 41.5'], calcareous.

@ 13.56 m - 13.96 m [44.5' - 45.8'], calcareous.

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

PL	Moisture Content - % Natural	LL
X	20	X

Client: Ohio Department of Transportation					Project: ATH-33-40.981					Job No. 9821-3200.00											
LOG OF: Boring R-97					Sta. 49+001.73, 105.42m Rt. of US 33 Mainline					Date Drilled: 3/21/00-3/23/00					STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40						
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		WATER OBSERVATIONS:	GRADATION						Moisture Content - %								
				Drive	Press		% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL						
15.24	231.76					Water seepage at: 0.91 m [3.0'] Water level at completion: dry (before coring) dry (after coring)															
							<b>DESCRIPTION</b>														
[50]	[760.4]					Hard light brown SANDSTONE, micaceous, slightly weathered.															
16.76																					
[55]																					
18.29																					
[60]																					
19.81																					
[65]																					
21.34																					
[70]																					
21.79	225.21																				
[71.5]	[738.9]																				
22.85																					

Core  
3.048  
m  
[120"]

Rec  
3.048  
m  
[120"]

RQD  
100%

Core  
3.048  
m  
[120"]

Rec  
3.048  
m  
[120"]

RQD  
99%

- @ 21.17 m [69.4'], horizontal fracture.
- @ 21.20 m [69.6'], horizontal fracture.
- @ 21.29 m - 21.34 m [69.8' - 70.0'], limestone lense.
- @ 21.34 m - 21.52 m [70.0' - 70.6'], calcareous.
- @ 21.55 m [70.7'], becomes weathered.

Medium hard brown CLAYSTONE, arenaceous, weathered, broken.

@ 21.98 m [72.1'], becomes dark gray and slightly weathered.





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF:		Boring R-97		Location:			Sta. 49+001.73, 105.42m Rt. of US 33 Mainline			Date Drilled: 3/21/00-3/23/00			STANDARD PENETRATION (N)				
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:						Blows per 0.30 m				
				Drive	Press		Water seepage at: 0.91 m [3.0'] Water level at completion: dry (before coring) dry (after coring)						GRADATION				
							% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Moisture Content - %				
							DESCRIPTION						PL	Natural	LL		
														X	●	X	
38.10 [125]	206.90 [685.4]		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 93%		Medium hard gray CLAYSTONE, broken fractures, calcareous, arenaceous, weathered, contains limestone clasts.  @ 38.59 m [126.6'], becomes pyritic.  @ 39.01 m [128.0'], fractured with clay infilling. @ 39.20 m [128.6'], healed fracture.										
39.62 [130]							@ 39.81 m [130.6'], color change to red gray and brown. @ 39.90 m [130.9'], low angle fracture with clay infilling.  @ 40.51 m [132.9'], low angle fracture with clay infilling. @ 40.84 m [134.0'], becomes very arenaceous.										
41.15 [135]			Core 3.048 m [120"]	Rec 2.972 m [117"]	RQD 91%												
42.67 [140]							@ 42.67 m - 46.94 m [140.0' - 154.0'], gas pockets encountered.										
43.13 [141.5]	203.87 [668.9]						@ 43.13 m [141.5'], clay seam.										
43.89 [144.0] 44.20 [145]	203.11 [666.4]		Core 3.048 m [120"]	Rec 3.023 m [119"]	RQD 95%		Hard gray SANDSTONE, micaceous, arenaceous, pyritic, weathered.  @ 43.74 m - 43.89 m [143.5' - 144.0'], vertical fracture.  Medium hard gray CLAYSTONE, micaceous, broken, slightly weathered to weathered, contains clay seams throughout.										
45.72																	

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

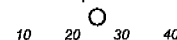
LOG OF: Boring R-97

Location: Sta. 49+001.73, 105.42m Rt. of US 33 Mainline

Date Drilled: 3/21/00-3/23/00

STANDARD PENETRATION (N)

Blows per 0.30 m

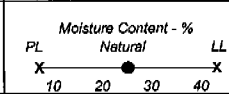


Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [in]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
45.72 [150]	201.28 [660.4]					
46.94 [154.0] 47.24 [155]	200.06 [656.4]					
48.77 [150]						
50.29 [155]						
51.82 [170]						
53.34						

WATER  
OBSERVATIONS:  
Water seepage at: 0.91 m [3.0']  
Water level at completion: dry (before coring)  
dry (after coring)

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay



**DESCRIPTION**

Medium hard gray CLAYSTONE, micaceous, broken, slightly weathered to weathered. contains clay seams throughout.

Bottom of Boring - 46.94 m [154.0']

PL	Moisture Content - %		LL
X	Natural		X
10	25		40







Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-99

Location: Sta. 49+267.17, 34.50m Rt. of US 33 Mainline

Date Drilled: 03/29/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	222.75					
0.15	222.60					
[0.5]	[730.3]					
0.61	222.14	4	.356	1A		
[2.0]	[728.8]	4	[14]	1B		
1.22	221.53	4	.229			
[4.0]	[726.8]	50/13	[9]	2		
1.52						
1.68	221.07					
[5.5]	[725.3]					
		50/10	.102			
			[4]	3		
		50/10	.102			
			[4]	4		
3.05	219.70					
[10]	[720.8]	Core 1.219 m [48"]	Rec 1.092 m [43"]	RQD 90%		
4.57		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%		
[15]						
6.10						
[20]						
7.62						

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: dry (before coring)  
 5.24 m [17.2] (after coring)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
8	19	—	31	29	14

**DESCRIPTION**

Topsoil - 0.152 m [6"]

Verv stiff brown SILT AND CLAY (A-6a), trace fine to coarse sand; micaceous; organic with rootlets; damp.

Loose brown COARSE AND FINE SAND (A-3a), some siltv clay; micaceous; damp.

Soft light brown and gray SHALE, arailaceous, decomposed.

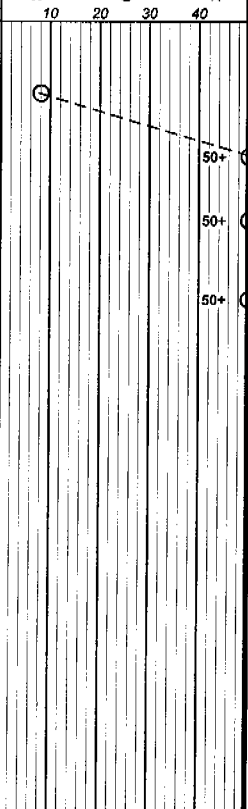
Soft light brown, fine to coarse grained SANDSTONE, arailaceous, micaceous, severely weathered.

Medium hard brown and light gray SANDSTONE, micaceous, weathered.

@ 5.46 m [17.9], horizontal fracture.

@ 6.31 m [20.7], horizontal fracture.

@ 6.43 m [21.1], 0.05' clay seam.





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring R-99

Location: Sta. 49+267.17, 34.50m Rt. of US 33 Mainline

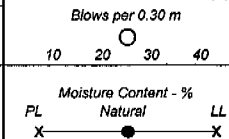
Date Drilled: 03/29/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: dry (before coring)  
 5.24 m [17.2'] (after coring)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay



15.24 207.51

**DESCRIPTION**

[50]	[680.8]	Core 1.524 m [60"]	Rec 1.473 m [58"]	RQD 42%
16.76		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 70%
[55]				
17.98	204.77			

Soft to medium hard gray CLAYSTONE, slightly micaceous, pyritic, weathered to severely weathered, contains limestone clasts

[59.0] [671.8]

Soft to medium hard red and gray CLAYSTONE, weathered

[60]		Core 1.524 m [60"]	Rec 1.448 m [57"]	RQD 95%
19.81		Core 1.524 m [60"]	Rec .813 m [32"]	RQD 53%

@ 18.93 m - 19.00 m [62.1' - 62.3'] gray.

@ 19.51 m [64.0]', becomes red, gray, and brown intermixed.

[70] [659.8]

Medium hard gray SILTSTONE, broken, weathered to slightly weathered.

[71.0]	201.11 [659.8]	Core 1.524 m [60"]	Rec 1.448 m [57"]	RQD 45%
21.34				
[70]				
21.64				
[71.0]				
22.86				

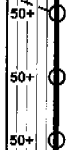
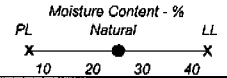
@ 22.56 m [74.0]', slightly broken.



Client: Ohio Department of Transportation Project: ATH-33-40.981 Job No. 9821-3200.00

LOG OF: Boring R-100 Location: Sta. 49+265.95, 101.88m Rt. of US 33 Mainline Date Drilled: 3/23/00 & 3/24/00 STANDARD PENETRATION (N)  
Blows per 0.30 m

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: none (before coring) 10.97 m [36.0'] (after coring)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m			
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Moisture Content - % Natural		LL
0	232.87						<b>DESCRIPTION</b>										
0.34	232.53						Topsoil - 0.33 m [13"]										
[1.1]	[762.9]	2					Hard brown SILT AND CLAY (A-6a), trace fine to coarse sand; damp.										
		2	.330														
0.91	231.96						Soft light brown SILTSTONE, argillaceous, micaceous, severely weathered to decomposed.										
[3.0]	[761.0]	5	[13]														
		24	.406				@ 1.83 m [6.0'], becomes severely weathered.										
1.52		50/10	[16]														
		29	.203														
[5]		50/05	[8]														
		50/13	.127														
		[5]															
3.05	229.82						Medium hard brown SILTSTONE, micaceous, arenaceous, weathered to severely weathered.										
[10]	[754.0]	Core 1.219 m [48"]	Rec .991 m [39"]														
							@ 3.20 m - 3.29 m [10.5' - 10.8'], high angle, rust stained fracture. @ 3.81 m [12.5'], clay seam. @ 3.87 m - 3.93 m [12.7' - 12.9'], horizontal fracture. @ 4.39 m - 4.45 m [14.4' - 14.6'], gray.										
4.54	228.33																
[14.9]	[749.1]	Core 3.048 m [120"]	Rec 3.048 m [120"]				Hard brown SANDSTONE, micaceous, very thin bedded, slightly broken, weathered. @ 4.69 m - 4.82 m [15.4' - 15.8'], gray. @ 4.75 m - 4.82 m [15.6' - 15.8'], claystone layer. @ 5.03 m - 5.12 m [16.5' - 16.8'], gray. @ 5.58 m - 5.61 m [18.3' - 18.4'], high angle fracture.										
5.70	227.17						Medium hard gray SILTSTONE with claystone interbeds, slightly micaceous, weathered. @ 6.07 m - 6.22 m [19.9' - 20.4'], sandstone layers. @ 6.46 m - 6.55 m [21.2' - 21.5'], severely weathered. @ 7.07 m - 7.13 m [23.2' - 23.4'], severely weathered.										
[18.7]	[745.3]																
							@ 7.41 m [24.3'], claystone, severely weathered.										
7.62																	



**Client:** Ohio Department of Transportation

**Project:** ATH-33-40.981

**Job No.** 9821-3200.00

**LOG OF:** Boring R-100

**Location:** Sta. 49+265.95, 101.88m Rt. of US 33 Mainline

**Date Drilled:** 3/23/00 & 3/24/00

**STANDARD PENETRATION (N)**

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
7.62	225.25					
[25]	[739.0]	Core 3.048 m	Rec 3.023 m	RQD	99%	
8.05	224.82					
[26.4]	[737.6]	[120"]	[119"]			
9.14						
[30]						
9.69	223.18					
[31.8]	[732.2]					
10.67		Core 3.048 m	Rec 3.048 m	RQD	94%	
[35.5]	[728.5]	[120"]	[120"]			
11.40	221.47					
[37.4]	[726.6]					
12.19						
[40]						
13.72		Core 3.048 m	Rec 3.048 m	RQD	95%	
[45]		[120"]	[120"]			
15.24						

**WATER OBSERVATIONS:**  
 Water seepage at: none  
 Water level at completion: none (before coring)  
 10.97 m [36.0] (after coring)

**GRADATION**

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Blows per 0.30 m  
 10 20 30 40  
 Moisture Content - %  
 PL Natural LL  
 X ————— X

**DESCRIPTION**

Soft brown CLAYSTONE, severely weathered

Hard light gray SANDSTONE, fine to coarse grained, calcareous, slightly micaceous, weathered to slightly weathered.  
 @ 9.30 m [30.5]', becomes noncalcareous.

Hard brown and gray SANDSTONE, fine to medium grained, poorly cemented, slightly broken with rust staining.  
 @ 9.85 m [32.3]', void encountered, lost water return.

Soft to medium hard brown SILTSTONE with clay interbeds, severely weathered.

Hard light gray and brown, SANDSTONE, fine to medium grained, poorly cemented, micaceous, weathered.  
 @ 13.41 m - 23.01 m [44.0' - 75.5]', no water return.  
 @ 14.02 m - 14.36 m [46.0' - 47.1]', contains interbedded black carbonaceous claystone, broken and fractured, severely weathered.  
 @ 14.36 m [47.1]', color change to brown.









Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF:		Boring R-101		Location:		Sta. 49+379.86, 64.96m Rt. of US 33 Mainline		Date Drilled: 3/30/00		STANDARD PENETRATION (N)			
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	GRADATION					Blows per 0.30 m	
				Drive	Press		% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	10
WATER OBSERVATIONS:												Moisture Content - %	
Water seepage at: none												PL	
Water level at completion: none (before coring)												Natural	
0.76 m [2.5'] (after coring)												LL	
												X	
												X	
0	218.75						<b>DESCRIPTION</b>						
0.37	218.38						Topsoil - 0.356 m [14"]						
[1.2]	[716.5]	1					Medium dense brown COARSE AND FINE SAND (A-3a), some silty clay; micaceous; damp.						
0.91	217.84	4	.330		1		Soft brown and gray SANDSTONE, micaceous, severely weathered to decomposed.						
[3.0]	[714.7]	7	[13]										
1.52		40	.203		2								
[5]		50/05	[8]										
1.52		50/08	.076		3								
[5]		[3]											
3.05	215.70						Medium hard brown and gray SANDSTONE, poorly cemented, weathered @ 3.41 m - 3.51 m [11.2' - 11.5'], high angle fracture.						
[10]	[707.7]	Core 1.219 m [48"]	Rec 1.219 m [48"]		RQD	77%							
4.48	214.27						@ 3.99 m - 4.08 m [13.1' - 13.4'], highly broken zone.						
[14.7]	[703.0]	Core 1.524 m [60"]	Rec 1.422 m [56"]		RQD	90%	Soft to medium hard dark gray SILTSTONE, argillaceous, broken, weathered, @ 4.82 m - 4.97 m [15.8' - 16.3'], high angle fracture with rust staining.						
[15]													
5.79	212.96						Soft brown SANDSTONE, fine grained, severely weathered to weathered.						
[19.0]	[698.7]	Core 1.524 m [60"]	Rec .711 m [28"]		RQD	12%							
6.10													
[20]													
6.86	211.89						Soft gray SILTSTONE, argillaceous, contains carbonaceous layers, severely weathered.						
[22.5]	[695.2]												
7.32	211.43						Soft black COAL, weathered to severely weathered.						
[24.0]	[693.7]												
7.62													





Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring SB-26

Location: Sta. 48+860.00, 19.50 m left of US 33 centerline Date Drilled: 2/1/00

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL X

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	201.66					
0.20	201.46 [660.9]					
0.7				7	229	1
				5	191	
1.52				3	305	2
				4	1121	
[5]				2		3
				3	330	
				4	1131	1
3.05				1	381	2
				2	1151	
[10]						4
3.66	198.00 [649.6]			3	381	5A
				20	1151	
[12.0]						5B
4.42	197.24			30	127	6
				50/00	151	
4.57	[647.1]			Core	Rec	R&D
4.63	[152.3]			0.533 m [21"]	533 m [21"]	100%
5.18	196.46 [644.6]			Core	Rec	R&D
				3.046 m [120"]	3.046 m [120"]	100%
[17.0]						
6.10						
[20]						
7.62						

WATER OBSERVATIONS:  
 Water seepage at: 1.86 m [6.1'], 1.92 m [6.3']  
 Water level at completion: dry prior to core  
 1.07 m [3.5']

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SR	% Clay

DESCRIPTION

Topsoil - 0.203 m [8"]

Very stiff dark brown SILT AND CLAY (A-6a), little fine to coarse sand; moist.  
 @ 1.07 m - 1.52 m [3.5' - 5.0'], becomes stiff, brown.

@ 1.83 m - 3.81 m [6.0' - 12.0'], soft, gray, with some fine to coarse sand.

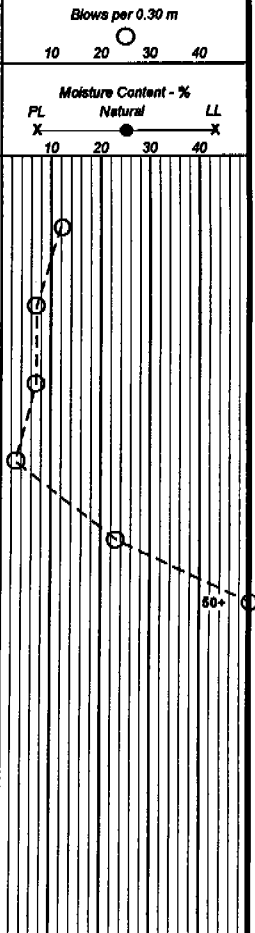
@ 2.59 m - 3.66 m [8.5' - 12.0'], contains few coal fragments.

Severely weathered gray SHALE, micaceous.

Medium hard gray fine grained SANDSTONE, micaceous; weathered.

Hard light gray LIMESTONE, micaceous, arenaceous; weathered to slightly weathered.

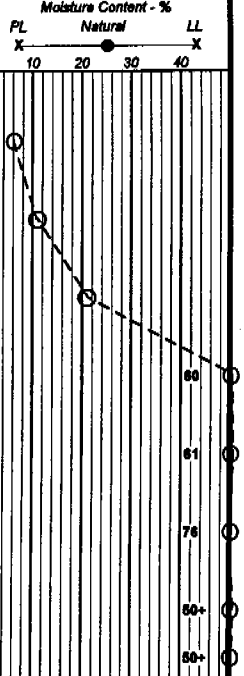
Medium hard to hard gray fine to medium grained SANDSTONE, micaceous; weathered to slightly weathered.







Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00										
LOG OF: Boring SB-29		Location: Sta. 48+940.13, 7.11 m left of US 33 centerline				Date Drilled: 2-8-00				STANDARD PENETRATION (N) Blows per 0.30 m								
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 3.51 m [11.5'] Water level at completion: dry prior to coring 2.13 m [7.0'] after coring	GRADATION						Moisture Content - % PL Natural LL				
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SR	% Clay		10	20	30	40
<b>DESCRIPTION</b>																		
0	209.36 [686.9]						Topsoil - 0.406 m [16"]											
0.40	208.96 [685.6]	2					Stiff brown SILT AND CLAY (A-6a); damp.											
[1.3]	[685.6]		2	.305 [12]	1	192 [2.0]												
0.91	208.45 [683.9]						Hard brown and red SILTY CLAY (A-6b); (decomposed shale); damp.											
[3.0]	[683.9]	2	3	.229 [9]	2	431+ [4.6+]												
1.62							Hard red CLAY (A-7-6); (decomposed claystone); damp.											
[5]		8	8	.229 [9]		431+ [4.5+]												
1.83	207.53 [680.9]						Soft brown SHALE; severely weathered.											
[6.0]	[680.9]	6	10	.229 [9]	3													
2.44	206.92 [678.9]						Soft purple and brown CLAYSTONE; severely weathered.											
[8.0]	[678.9]	17	27	.381 [15]	4													
3.05	206.16 [678.4]						Soft to medium hard gray SHALE, arenaceous; severely weathered.											
[10.5]	[678.4]	15	18	.305 [12]	5													
3.98	205.40 [673.9]						Soft to medium hard gray SHALE, arenaceous; severely weathered.											
[13.0]	[673.9]	27	29	.279 [11]	6													
4.57							@ 5.64 m [18.5'], becomes weathered.											
[15]		18	29	.356 [14]	7													
6.10	203.26 [666.9]						@ 6.10 m - 7.32 m [20.0' - 24.0'], slightly broken.											
[20]	[666.9]	Core	Rec															
		1.219 m [48"]	635 m [25"]				@ 6.77 m - 6.98 m [22.2' - 22.9'], vertical fracture with rust staining.											
7.41	201.95 [662.6]						Soft to medium hard red CLAYSTONE; weathered.											
[24.3]	[662.6]																	



Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00															
LOG OF: Boring SB-29				Location: Sta. 48+940.13, 7.11 m left of US 33 centerline				Date Drilled: 2-8-00															
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.30 m	Rec (m) [ft]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:  Water seepage at: 3.51 m [11.5'] Water level at completion: dry prior to coring 2.13 m [7.0'] after coring	GRADATION															
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SR	% Clay	STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40  Moisture Content - % PL Natural LL X ● X 10 20 30 40									
7.62 [25]	201.74 [681.9]																						
8.02 [26.3]	201.34 [660.8]																						
9.14 [30]																							
9.94 [32.6]	199.42 [654.3]																						
10.36 [34.0]	199.00 [652.9]																						
10.67 [35]																							
12.19 [40]																							
13.72 [45]																							
15.24																							

**DESCRIPTION**

Soft to medium hard red CLAYSTONE; weathered.

Medium hard gray SHALE, arenaceous; weathered.

@ 8.84 m - 9.94 m [29.0' - 32.6'], weathered to severely weathered.

Soft to medium hard red CLAYSTONE; weathered to severely weathered.

Bottom of Boring - 10.36 m [34.0']









Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

LOG OF: Boring SB-34

Location: Sta. 49+180.00, on US-33 centerline

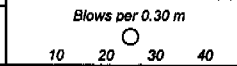
Date Drilled: 2/14/00

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m)	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	

WATER  
OBSERVATIONS:  
Water seepage at: 1.37 m [4.5'], 3.84 m [12.6']  
Water level at completion: 1.22 m [4.0'] (before and after  
coring)

GRADATION



7.62 204.69

**DESCRIPTION**

[25]	[671.6]	Core 1.524 m [60"]	Rec 1.422 m [56"]	RQD 88%
------	---------	-----------------------------	----------------------------	------------

Soft to medium hard gray, brown red, and black CLAYSTONE, broken, arenaceous; severely weathered.

@ 9.51 m - 10.09 m [31.2' - 33.1']. contains several slickensided shears.

[30.0]		Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 100%
--------	--	-----------------------------	----------------------------	-------------

[33.4]	202.13 [663.2]			
--------	-------------------	--	--	--

[35]		Core 3.048 m [120"]	Rec 3.048 m [120"]	RQD 100%
------	--	------------------------------	-----------------------------	-------------

Medium hard gray SANDSTONE, argillaceous; severely weathered.

[40]				
------	--	--	--	--

[45.0]	198.72 [652.0]			
--------	-------------------	--	--	--

Bottom of Boring - 13.59 m [44.6']

--	--	--	--	--

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

PL	Natural	LL
X	●	X

Client: Ohio Department of Transportation

Project: ATH-33-40.981

Job No. 9821-3200.00

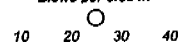
**LOG OF: Boring SB-35**

Location: Sta. 48+676.95, on US-33 centerline

Date Drilled: 2/21/00

STANDARD PENETRATION (N)

Blows per 0.30 m



Moisture Content - %



Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	207.93 [682.2]					
0.38 [1.3]	207.55 [680.9]	WOH	.229 [9]			48 [0.5]
0.91 [3.0]	207.02 [679.2]	WOH				
1.52 [5]		1	.356 [14]			
1.92 [6.3]	206.01 [675.9]	2				
2.44 [8.0]	205.49 [674.2]	9	.279 [11]			239 [2.5]
3.05 [10.5]	204.73 [671.7]	13	.254 [10]			
3.20 [10.5]	204.73 [671.7]	50/13				
4.57 [15]		42	.406 [16]			
4.97 [16.3]	202.96 [665.9]	47	.229 [9]			431+ [4.5+]
6.10 [20]		9	.229 [9]			
7.62 [25]		16	.229 [9]			

WATER OBSERVATIONS:  
 Water seepage at: 1.07 m [3.5] - 1.92 m [6.3]  
 Water level at completion: dry

GRADATION

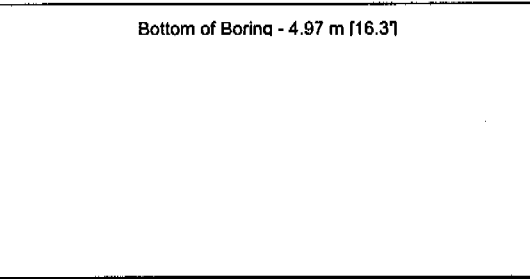
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

DESCRIPTION

Depth (m) [ft]	Description
0 - 0.38	Topsoil - 0.381 m [15"]
0.38 - 0.91	Soft gray SILT AND CLAY (A-6a); slightlv organic; moist.
0.91 - 1.52	Very loose gray SANDY SILT (A-4a). trace gravel; moist to wet.
1.52 - 2.44	Very stiff gray and brown SILTY CLAY (A-6b). trace fine to coarse sand. trace gravel; (decomposed shale); damp.
2.44 - 3.05	Soft gray SILTSTONE. arenaceous; severely weathered.
3.05 - 4.57	Soft gray SHALE; severely weathered to weathered.
4.57 - 4.97	
4.97 - 6.10	
6.10 - 7.62	

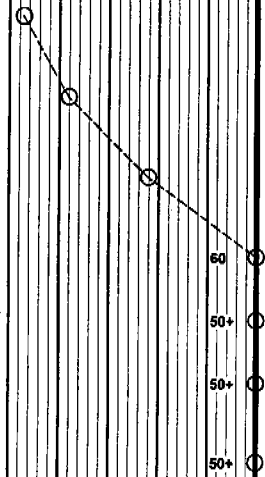
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Bottom of Boring - 4.97 m [16.3']





Client: Ohio Department of Transportation				Project: ATH-33-40.981				Job No. 9821-3200.00								
LOG OF:		Boring SB-36		Location: Sta. 48+302.40, on US-33 centerline				Date Drilled: 2/21/00				STANDARD PENETRATION (N) Blows per 0.30 m				
Depth (m) (ft)	Elev. (m) (ft)	Blows per 0.15 m	Rec (m) [in]	Sample No.		Hand Penetro- meter (kN/m <sup>2</sup> ) [tsf]	GRADATION						Moisture Content - %			
				Drive	Press		% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	PL	Natural	LL	
0	221.33						<b>DESCRIPTION</b>						X ————— X			
0.25	221.08						Topsoil - 0.254 m [10"]									
[0.8]	[725.3]			WOH 2	.127 [5]		Very loose brown SANDY SILT (A-4a); contains organic material; damp to moist.									
0.91	220.42			8			Medium dense brown and gray COARSE AND FINE SAND (A-3a), trace clayey silt; damp to moist.									
[3.0]	[723.2]			7	.203 [8]											
1.52				5												
[5]				1												
2.13	219.20			1	.229 [9]		Severely weathered brown SANDSTONE, micaceous, argillaceous.									
[7.0]	[719.2]			27												
3.05				13												
[10]				10	.356 [14]											
				50												
				23	.203 50/.08 [8]											
				50/.13	.127 [5]											
4.57																
[15]																
4.94	218.39			50/.05	.051 [2]		Bottom of Boring - 4.94 m [16.2']									
[16.2]	[709.9]			2												
6.10																
[20]																
7.62																



Client: ODOT District 10				Project: ATH-33-40.891 (US 33 over Pratts Fork)				Job No. 9821-3200.00					
LOG OF: Boring SB-7				Location: 39+650.2, 24.7 m Lt. of US 33 centerline				Date Drilled: 11/8/99					
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:	GRADATION					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
							Water seepage at: none	STANDARD PENETRATION (N) Blows per 0.30 m					
							Water level at completion: dry prior to coring	10 20 30 40					
							7.32 m [24.0'] (with core water)	Moisture Content - % PL Natural LL					
							X ————— X						
							10 20 30 40						
0	258.23						<b>DESCRIPTION</b>						
0.34	257.89						Topsoil - 0.33 m [13"]						
(1.1)	(846.1)	5				1	431+ (4.5+)						
		6	.330				Hard brown SILT AND CLAY (A-8a); decomposed claystone; damp.						
		12	[13]										
		6					431+ (4.5+)						
		8	.152			2							
1.52	256.71						Hard gray SANDSTONE, fine grained, micaceous; slightly weathered.						
(5)	(842.2)	Core	Rec			RQD	Medium hard brown SANDSTONE, fine to medium grained, micaceous; weathered; friable. @ 2.19 m - 2.32 m [7.2' - 7.6'], decomposed layer.						
1.80	256.43	0.61 m	.381 m			42%							
(5.9)	(841.3)	24"	15"				@ 2.56 m - 2.59 m [8.4' - 8.5'], vertical fracture. @ 2.71 m - 2.77 m [8.9' - 9.1'], clay seam.						
		Core	Rec			RQD							
		1.524 m	1.448 m			52%	Hard gray SANDSTONE, fine grained, micaceous; slightly weathered.						
		[60"]	[67"]										
2.90	255.33						Hard gray LIMESTONE; moderately weathered. @ 4.42 m - 4.57 m [14.5' - 15.0'], contains fine to medium sand.						
(9.5)	(837.7)												
		Core	Rec			RQD	Soft to medium hard brown SANDSTONE; fine to medium grained, severely weathered. @ 5.55 m - 5.61 m [18.2' - 18.4'] & 6.22 m - 6.43 m [20.4'-21.1'] clayey decomposed layers.						
		1.524 m	1.346 m			68%							
4.05	254.16						@ 5.76 m [18.9], 6.52 m [21.4'], 6.71 m [22.0'], clayey seams.						
(13.3)	(833.9)	[60"]	[53"]										
4.57							@ 6.71 m - 6.77 m [22.0' - 22.2'], severely weathered.						
(15)													
4.85	253.38						Soft to medium hard brown SANDSTONE; fine to medium grained, severely weathered. @ 5.55 m - 5.61 m [18.2' - 18.4'] & 6.22 m - 6.43 m [20.4'-21.1'] clayey decomposed layers.						
(15.9)	(831.3)												
		Core	Rec			RQD	@ 5.76 m [18.9], 6.52 m [21.4'], 6.71 m [22.0'], clayey seams.						
		1.524 m	1.321 m			35%							
		[60"]	[52"]				@ 6.71 m - 6.77 m [22.0' - 22.2'], severely weathered.						
6.10							Soft to medium hard brown SANDSTONE; fine to medium grained, severely weathered. @ 5.55 m - 5.61 m [18.2' - 18.4'] & 6.22 m - 6.43 m [20.4'-21.1'] clayey decomposed layers.						
(20)													
		Core	Rec			RQD	@ 6.71 m - 6.77 m [22.0' - 22.2'], severely weathered.						
		1.524 m	1.397 m			28%							
		[60"]	[55"]				Soft to medium hard brown SANDSTONE; fine to medium grained, severely weathered. @ 5.55 m - 5.61 m [18.2' - 18.4'] & 6.22 m - 6.43 m [20.4'-21.1'] clayey decomposed layers.						
7.52							Soft to medium hard brown SANDSTONE; fine to medium grained, severely weathered. @ 5.55 m - 5.61 m [18.2' - 18.4'] & 6.22 m - 6.43 m [20.4'-21.1'] clayey decomposed layers.						



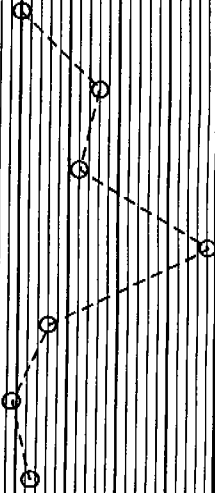


**LOG OF: Boring SB-8** Location: 39+683.1, 5.5 m Lt. of US 33 centerline Date Drilled: 11/10/99

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: dry prior to coring 1.15 m [3.8'] (with core water)	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40
				Drive	Press			% Aggregate	% C. Sand	% F. Sand	% S.F. Sand	% SR	% Clay	

							<b>DESCRIPTION</b>													
7.62 [25]	231.01 [757.9]	Core 1.524 m [60"]	Rec 1.473 m [58"]	ROD 80%			Medium hard red and gray CLAYSTONE; weathered.													
8.02 [26]	230.61 [756.6]						Medium hard gray SILTY CLAYSTONE, slightly calcareous; weathered. @ 8.44 m [27.7'], slickensided shear.													
8.69 [28]	229.94 [754.4]						Medium hard gray SILTSTONE, slightly calcareous; weathered; contains limestone clasts.													
9.14 [30]	229.49 [752.6]						Bottom of Boring - 9.14 m [30.0']													
10.67 [35]																				
12.18 [40]																				
13.72 [45]																				
15.24																				

Client: ODOT District 10				Project: ATH-33-40.891 (US 33 over Pratts Fork)				Job No. 9821-3200.00					
LOG OF: Boring SB-9				Location: 39+721.5, 19.8 m Lt. of US 33 centerline				Date Drilled: 11/10/99					
Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrator (kNm <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: none Water level at completion: dry prior to coring 1.14m [3.75'] (with core water)	GRADATION					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% S#	% Clay
							STANDARD PENETRATION (N) Blows per 0.30 m 10 20 30 40 Moisture Content - % PL Natural LL X X						
0	234.98						<b>DESCRIPTION</b>						
0.23	234.75 [770.2]						Topsoil removed by dozer ~ 0.229 m [9"]						
			4	.432		1	311 [3.25]	Very stiff brown SILTY CLAY (A-6b), trace to little fine to coarse sand; damp.					
			6	[17]									
1.07	233.91 [767.4]		11	.457		2	431+ [4.5+]	Hard brown and light brown SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; ; damp.					
			15	[18]									
1.52													
1.83	233.15 [764.9]		10	.408		3		Medium dense dark brown GRAVEL WITH SAND, SILT, AND CLAY (A-2-6); damp.					
			12	[16]									
			16										
3.05			13	.381		4		@ 2.59 m [8.5], dense and light brown.					
			35	[15]									
3.51	231.47 [759.4]		8	.432		5	120 [1.25]	Stiff brown SILTY CLAY (A-6b); contains fine sand seams; moist.					
			8	[17]									
4.11	230.87 [757.4]		4	.330		6		Loose light brown, dark brown and gray GRAVEL WITH SAND, SILT, AND CLAY (A-2-6); damp to moist.					
			5	[13]									
4.57													
			3										
5.18	229.80 [753.9]		4	.432		7		Hard purple, brown and gray SILTY CLAY (A-6b); severely weathered claystone; damp.					
			9	[17]									
6.10													
6.40	229.58 [749.9]		Core 3.048 m [120"]	Rec 2.794 m [110"]	RQD 75%			Soft to medium hard gray SILTY CLAYSTONE; weathered. @ 6.40 m - 7.01 m [21.0' - 23.0'], slightly calcareous.  @ 7.41 m [24.3'], contains limestone clasts.  @ 7.62 m [25.0'], contains limestone clasts.					
7.62													





Client: ODOT District 10

Project: ATH-33-40.891 (US 33 over Pratts Fork)

Job No. 9821-3200.00

LOG OF: Boring SB-10

Location: 39+743.0, 5.5 m Lt. of US 33 centerline

Date Drilled: 11/11/99

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL X

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS:	GRADATION						STANDARD PENETRATION (N) Blows per 0.30 m	Moisture Content - %					
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay							
0	231.30																			
0.30	231.00						<b>DESCRIPTION</b>													
(1.0)	[757.9]	2					Topsoll - 0.305 m [12"]													
0.61	230.89		.432			1	Very stiff dark brown SILT (A-4b), trace fine sand; slightly organic; damp.													
(2.0)	[756.9]	4	[1.7]				Medium dense brown COARSE AND FINE SAND (A-3a), trace silty clay, trace gravel; damp.													
1.07	230.23						Hard brown, gray and black SILTY CLAY (A-6b), trace fine to coarse sand; damp.													
(3.5)	[755.3]	6				2														
1.52		5	.457				Medium dense brown COARSE AND FINE SAND (A-3a), some clayey silt, trace gravel; damp to moist.													
(5)		5	[1.8]			3														
1.83	229.47						Soft brown and gray SILTY CLAY (A-6b), little fine to coarse sand, trace gravel; moist.													
(6.0)	[752.9]	5	.381				@ 2.62 m - 2.68 m [8.6' - 8.8'], coal and carbonaceous shale fragments.													
		5	[1.5]			4														
2.59	228.71						Medium dense brown and red GRAVEL WITH SAND (A-1-b); moist to wet.													
(8.5)	[750.4]	2	.457				Hard gray SILTY CLAY (A-6b); decomposed silty shale; damp.													
3.05		2	[1.8]																	
(10)		4					@ 5.64 m [18.5'], becomes severely weathered silty shale.													
3.51	227.79																			
(11.5)	[747.3]	4	.406																	
		4	[1.6]																	
4.11	227.19																			
(13.5)	[745.4]	6																		
4.57		21	.432																	
(15)		14	[1.7]																	
		14																		
		31	.457																	
		36	[1.8]																	
		40	.229																	
		50/08	[.9]																	
6.10	225.20																			
(20)	[738.8]	Core	3.048	Rec	2.794		Soft gray SHALE; severely weathered.													
		m	[10']	m	[110"]															
6.52	224.78						Hard gray SANDSTONE, fine to medium grained, micaceous; slightly weathered.													
7.62																				

67

60+







Client: ODOT District 10

Project: ATH-33-40.891 (US 33 over Pratts Fork)

Job No. 9821-3200.00

LOG OF: Boring SB-12

Location: 39+803.2, 5.4 m Lt. of US 33 centerline

Date Drilled: 11/1/99

STANDARD PENETRATION (N)

Blows per 0.30 m

10 20 30 40

Moisture Content - %

PL Natural LL

10 20 30 40

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]	WATER OBSERVATIONS: Water seepage at: 1.22 m [4.0'] Water level at completion: 0.76 m [2.5'] (before coring) 2.68 m [8.8'] (after coring)	GRADATION						STANDARD PENETRATION (N)		Moisture Content - %		
				Drive	Press			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SH	% Clay	PL	Natural	LL		
0	229.42																	
0.37	229.05 (751.5)																	
(1.2)																		
0.91	228.51 (749.7)																	
(3.0)																		
1.52																		
(5)																		
1.83	227.59 (746.7)																	
(6.0)																		
3.05																		
(10)																		
4.18	225.24																	
4.27	225.15 (738.7)																	
4.57																		
(15)																		
4.91	224.51 (736.6)																	
6.10																		
(20)																		
7.62																		

## DESCRIPTION

Topsoil - 0.356 m [14"]

Stiff to very stiff brown SILT AND CLAY (A-6a), some fine to coarse sand; moist.

Medium dense brown GRAVEL WITH SAND AND SILT (A-2-4), trace to little clay; damp.

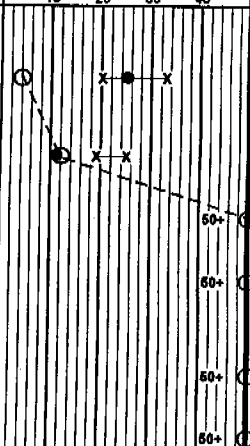
Hard gray SANDY SILT (A-4a); decomposed to severely weathered siltstone; damp.

Hard gray SANDSTONE; contains a vertical fracture.  
Soft gray CLAYSTONE; arenaceous; weathered.Hard gray SANDSTONE, micaceous; slightly weathered.  
@ 5.12 m - 5.24 m [16.8' - 17.2'], calcareous.  
@ 5.55 m - 5.90 m [18.2' - 19.4'], calcareous.

@ 6.16 m - 6.36 m [20.2' - 20.9'], argillaceous.

@ 6.61 m - 6.71 m [21.7' - 22.0'], argillaceous.

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SH	% Clay
0	6	—	28	46	21
38	21	—	15	16	10



Client: ODOT District 10

Project: ATH-33-40.891 (US 33 over Pratts Fork)

Job No. 9821-3200.00

LOG OF: Boring SB-12

Location: 39+803.2, 5.4 m Lt. of US 33 centerline

Date Drilled: 11/1/99

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.30 m	Rec (m)	Sample No.		Hand Penetro- meter ( $kN/m^2$ ) [tsf]
				Drive	Press	
7.62 [25]	221.80 [727.7]					
8.44	220.88 [725.0]					
9.14 [30]						
10.67 [35]						
12.19 [40]						
13.72 [45]						
15.24						

WATER

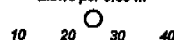
OBSERVATIONS:

Water seepage at: 1.22 m [4.0]  
 Water level at completion: 0.76 m [2.5] (before coring)  
 2.68 m [8.8] (after coring)

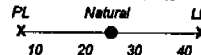
GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay

Blows per 0.30 m



Moisture Content - %



DESCRIPTION

Hard gray SANDSTONE, micaceous; slightly weathered.

⊙ 8.32 m - 8.40 m [27.3' - 27.5'], 45° fracture.  
 ⊙ 8.17 m - 8.44 m [26.8' - 27.7'], argillaceous.

Bottom of Boring - 8.44 m [27.7']



Client: ODOT District 10

Project: ATH-33-40.891 (US 33 over Pratts Fork)

Job No. 9821-3200.00

LOG OF: Boring SB-13

Location: 39+840.5, 16.8 m Lt. of US 33 centerline

Date Drilled: 11/1-11/4/99

STANDARD PENETRATION (M)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Rec (m) [ft]	Sample No.		Hand Penetra- meter (kJ/m <sup>2</sup> ) [tsf]
				Drive	Press	
7.62	226.70					
[25]	[743.8]					
			Core 1.524 m [60"]	Rec 1.397 m [55"]	RQD 45%	
9.14						
[30]						
9.57	224.75 [737.4]		Core 1.524 m [60"]	Rec 1.499 m [58"]	RQD 78%	
10.67			Core 1.524 m [60"]	Rec 1.524 m [60"]	RQD 97%	
[35]						
12.19						
[40]						
12.62	221.70					
12.80	221.52 [726.8]					
13.72						
[45]						
15.24						

WATER  
OBSERVATIONS:  
Water seepage at: none  
Water level at completion: dry prior to coring  
0.62 m [2.7'] (with core water)

GRADATION

% Aggregate	% C. Sand	% M. Sand	% F. Sand	% SR	% Clay

Blows per 0.30 m  
10 20 30 40

Moisture Content - %  
PL Natural LL  
X ————— X  
10 20 30 40

DESCRIPTION

Soft red CLAYSTONE; weathered.  
  
@ 8.99 m [29.5'], gray and red mottled.  
  
Medium hard gray SILTSTONE; moderately weathered.  
@ 9.66 m [31.7'], becomes calcareous.  
  
@ 10.67 m - 10.79 m [35.0' - 35.4'], partially healed fracture.  
@ 10.97 m - 11.06 m [36.0' - 36.3'], near vertical  
@ 11.09 m - 11.16 m [36.4' - 36.6'], limestone layer.  
@ 11.34 m [37.2'], clay seam.  
  
Hard gray SANDSTONE; calcareous; slightly weathered.  
  
Bottom of Boring - 12.80 m [42.0']







Client: ODOT District 10

Project: ATH-33-40.891 (US 33 over Pratts Fork)

Job No. 9821-3200.00

LOG OF: Boring SB-15

Location: 39+888.6, 19.8 m Lt. of US 33 centerline

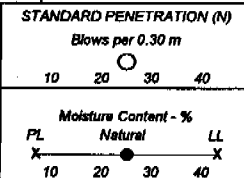
Date Drilled: 11/11/99

STANDARD PENETRATION (N)

Depth (m) [ft]	Elev. (m) [ft]	Blows per 0.15 m	Spec (m) [ft]	Sample No.		Hand Penetrometer (kN/m <sup>2</sup> ) [tsf]
				Drive	Press	
0	244.44					
0.43	244.01 [800.6]	2	.358 [1.41]			
1.52		24	.203 [0.8]			
		39	.229 [0.9]			
3.05		17	.358 [1.41]			
3.35	241.09					
3.47	240.97 [789.8]	32	.254 [1.0]	5A		
		50/10		5B		
4.57		24	.358 [1.41]			
4.88	239.58 [786.0]	22		7A		
		38	.432 [1.7]	7B		
5.64	238.80 [783.5]	17	.254 [1.0]			
5.85	238.59 [782.8]					
6.10						
6.40	238.04 [781.0]	Core	0.305	Rec	0%	
6.71	237.73 [778.5]	Core	1.524	Rec	55%	
6.88						
7.62						

WATER OBSERVATIONS:  
 Water seepage at: none  
 Water level at completion: dry prior to coring  
 1.37 m [4.5'] (with core water)

GRADATION					
% Aggregate	% C. Sand	% M. Sand	% F. Sand	% S.S.	% Clay



**DESCRIPTION**

Topsoll - 0.432 m [17"]

Medium dense to dense light brown GRAVEL WITH SAND (A-1-b); decomposed to severely weathered sandstone; damp.

Hard dark brown SILTY CLAY (A-6b); decomposed shale; damp  
 Hard brown and gray SILT AND CLAY (A-6a); decomposed siltstone; damp.

431+ [4.5+]

Hard black SILT AND CLAY (A-6a); decomposed carbonaceous shale with coal stringers; damp.

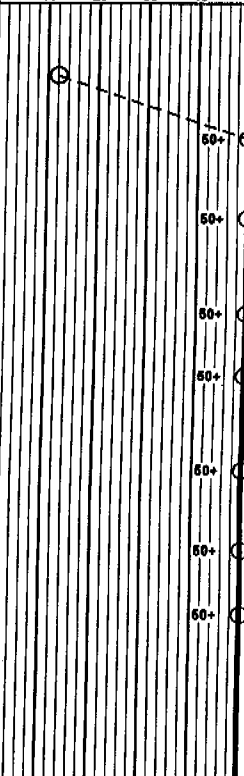
431+ [4.5+]

Hard gray SILTY CLAY (A-6b); decomposed shale; damp.

Very soft black COAL; CORE LOSS.

Soft dark gray SHALE, carbonaceous; weathered; LOSS.

Medium hard gray SHALE, calcareous; weathered; broken.  
 Hard gray LIMESTONE; moderately weathered.  
 @ 7.04 m - 7.19 m [23.1' - 23.6']; Soft SHALE, weathered.  
 @ 7.19 m - 7.38 m [23.6' - 24.2']; vertical fracture.  
 @ 7.41 m - 7.80 m [24.3' - 25.6']; Soft SHALE weathered.





# OHIO DEPARTMENT OF TRANSPORTATION



1980 West Broad Street  
P. O. Box 899  
Columbus, Ohio 43216-0899



Reply to:  
Office of Materials Management  
1600 West Broad Street, Columbus, Ohio 43223-1298

October 25, 2000

Dodson-Stilson, Inc.  
6121 Huntley Road  
Columbus, Ohio 43229-1003

Attention: Mr. Todd Willis, P.E., P.S.  
Project Manager

Re: ATH-33-40.981, PID 18288  
Stage 2 Geotechnical Review - 2<sup>nd</sup> Submittal

Gentlemen:

We have completed our review of the October 16, 2000 resubmittal of the subject project. Revised and marked-up cross-sections were included, as well as charts regarding the culvert undercuts.

Most of the work on the cross-sections appears to be done. There still may be work to be done at the following stations:

- 40+100 R Back & forward stas. have 4:1 exit slopes. A 3:1 slope is shown for this location.
- 41+860 R For four lane work, exit slope is drawn at 4:1, but labeled 3:1.
- 41+940, 960 Unsuitable material present. Should be shown 0.5m below proposed subgrade in cuts. We expect wet clays at proposed subgrade more than rock.
- 43+140, 160 Benches need to be shown.
- 43+520-560 The 10:1 on the left should be changed to a 2:1.
- 44+820 R A 4:1 should be used instead of the 10:1.
- 46+880-900 Undercut appears to be twice as thick as needed, or is this related to the culvert undercut?
- 48+100 R For four lane work, change the 4:1 to 3:1.
- 48+780 L Add 2:1 between 1/2:1 and 4:1.
- 49+560 R Show bench.
- 49+560-660 Bench areas were not cross-hatched as per the legend.

We have enclosed marked-up sheets as necessary.

T. Willis  
October 25, 2000  
Page 2

We agree with the proposed handling of the soft material at several of the culvert locations. Since the proposed undercuts are somewhat complicated in location and application, we recommend that a chart or table, such as what was provided to us, be included on the plans. Station limits, depth of removal, width of cuts, intent of removal, and quantities should be included. It would be very difficult to express these undercuts accurately in drawing form only.

It appears that there may be a slight difference in interpretation as to where the coal seam is located from Sta. 49+180 to 49+340. Since the intent is for the 10:1 bench to be located at the top of the coal seam in this area, a note should be included alerting the Contractor to this intent. They should be able to modify the bench location in the field as they uncover the coal seam.

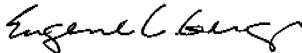
No plan notes were submitted. We assume that all necessary notes will be included in the plans.

Regarding the quantities for special benching, a little clarification may be needed. Showing actual bench location and dimensions on the cross-sections, and including Note R102, allows the Contractor to have quantities to bid for and get paid for, so we are more likely to get these benches performed compared to specification benching described in Item 203. The quantities of cut and fill should be included on the cross-sections to be carried to the General Summary.

Please resubmit corrected cross-sections along with any marked-up plans enclosed. Although we are asking for another submission, the Stage 2 Geotechnical Review is essentially complete.

Please feel free to contact us should you have any questions.

Respectfully,



Eugene C. Geiger, P.E.  
Geotechnical Engineering Coordinator

Encl.

ECG/SAS

cc: S. Eldabaja - G. Collins - J. Townley - S. Sommers - Reading File - ~~File~~

# MEMO

To: Brent Downing  
From: Dorothy Adams  
Subject: ATH-33-40.981, Culvert Recommendations  
Date: September 28, 2000



We have reviewed the letter from ODOT dated September 25, 2000 regarding the slope review submittal and the cross sections for the above-referenced project. Everything mentioned in the letter appears to be included in the current cross sections except for two items: 1) removal of unsuitable material, as mentioned on page 2 and 2) settlement concerns at selected culverts, as mentioned on page 3.

Regarding the removal of unsuitable material, in general, we concur with ODOT's recommendations. The existing soil should be removed to the specified depth below the proposed subgrade in cut sections and below the existing ground surface in fill sections. However, it should be noted that some of these areas traverse valleys with steep slopes where it may not be possible to overexcavate the specified amount of soil.

Two sections mentioned in the letter (Sta. 46+880 to 46+920 and Sta. 48+180 to 48+220) are located along proposed culverts which cross the mainline at angles of 37° and 45°. Because of the skew, some portions of these two culverts extend beyond the areas specified for overexcavation. Consequently, we have modified the areas to be overexcavated to include the entire valley along the culvert. These modifications are included in the attached table.

Regarding the concern about settlement of culverts under large fills, we have prepared the recommendations in the attached table. Rather than perform settlement analyses at each culvert location in question, we recommend overexcavation of any unsuitable or soft soils. The amount of overexcavation is generally 0.9 meters or less, which in many cases, will be required regardless of the soil conditions to place the pipes at the desired elevations.

Please note that a coal seam was encountered at the location of Culvert No. 34. The recommendations include removal of this coal seam where it is within one meter of the invert elevation.

Because of the variation in the existing grade, both excavation and fill placement will be necessary to construct the culverts. Excavation in rock is anticipated below depths of 2.0 to 2.5 meters. A granular material should be used for fill below the pipe, according to Item 603, ODOT Construction and Material Specifications.

For clarification, I confirmed with Steve Sommers at ODOT that the special benches do refer to the benches shown at the base of some of the proposed embankments where a side hill fill occurs. He suggested that if you call out the quantities for cut and fill for the special benches in the notes as we discussed, that this should be referenced on the cross sections as well.

Please feel free to call if you have any questions.

A handwritten signature in cursive script, appearing to read "Dorothy Adams".

ATH-33 40-981 Culvert Overexcavation Recommendations Due to Soft, Very Loose, or Unsuitable Soils  
(unless otherwise noted, the width of overexcavation is limited to the width of the pipe trench.)

Culvert Ref No.	Proposed Centerline Station	Boring No.	Elevation	Station	Offset	Recommendations (overexcavation below existing ground surface, as measured along pipe, L = left, R = right of centerline)
24	40+394.806	SB-53	226.64	40+457.61	71.05 m L	0.9 m, entire length of pipe
24	40+394.806	SB-52	229.91	40+344.43	84.67 m R	0.9 m, entire length of pipe
25	40+884.113	SB-51	239.03	40+884.10	CL	1.5 m, from 26 m right of CL to right end of pipe
28	42+116.376	B-99	256.00	42+086.97	4.97 m L	0.5 m, entire length of pipe
29a	42+768.328	SB-50	244.77	42+765.34	0.62 m L	none
30	43+039.346	SB-49	247.12	43+054.84	3.71 m L	none
31	43+776.488	SB-46	225.29	43+776.49	CL	2.4 m, entire length of pipe
48	43+940.776	SB-45	241.42	43+927.02	2.77 m L	none
32	44+415.818	SB-44	232.29	44+420.41	6.09 m R	none
33	44+939.506	SB-43	223.53	44+946.93	1.82 m R	none
34	45+305.668	SB-42	215.38	45+305.24	4.40 m R	overexcavate coal seam to elevation 212.0 between 10 m L and 37 m R
35	46+023.462	SB-41	220.91	46+020.87	CL	none
36	46+904.720	SB-40	238.84	46+898.31	4.16 m L	for embankment: 0.9 m between Sta. 46+880 and 46+920, as specified by ODOT for culvert: 0.9 m, entire length pipe
37	47+140.528	SB-39	233.83	47+143.36	1.12 m R	none
38	47+608.921	SB-38	223.80	47+610.18	2.09 m R	none
47	47+748.890	SB-37	225.52	47+749.13	0.27 m L	none
40	48+142.751	B-149	216.10	48+159.60	23.78 m R	0.5 m, between left end pipe and 24 m R, below elevation 216.0 (width varies, see plan)
40	48+142.751	R-88	214.20	48+194.71	34.22 m R	for embankment: 0.9 m between Sta. 48+180 and 48+220, as specified by ODOT for culvert: 2.4 m, between 24 m R and right edge embankment, below elevations 214.0 to 216.0 (width varies, see plan)
41	48+302.124	SB-36	221.33	48+302.40	CL	0.9 m, entire length of pipe
43	48+674.532	SB-35	207.93	48+676.95	CL	for embankment: 1.2 m between Sta. 48+640 and 48+700, as specified by ODOT for culvert: 1.7 m, entire length of pipe
44	49+173.478	SB-34	212.31	49+180.00	CL	none

Please note that due to the variations in the existing grade, some excavation and fill is expected to construct the pipe, regardless of the above recommendations. All topsoil, generally less than 0.3 meters thick, should be stripped prior to placement of the pipe. Excavation in rock is generally anticipated below depths of 2.0 to 2.5 meters.

**OVEREXCAVATION QUANTITIES**  
 TO BE ADDED TO THE GENERAL SUMMARY

Culvert No.	Excavation Dimensions			
	Width	Depth	Length	Volume
24	2.75	0.9	156.1695	386.5195
25	1.85	1.5	29.5147	81.90329
28	1.85	0.5	138.646	128.2476
31	2.575	2.4	265.9378	1643.496
34*	1.876		174.6379	327.6207
36	1.35	0.9	126.1102	153.2239
40LP		0.5	728.16	364.08
40RP		2.4	1355.91	3254.184
40SR		0.9	2940.23	2646.207
41	1.35	0.9	86.5	105.0975
43SR		1.2	4359.52	5231.423
43	1.7	1.7	129.6	374.544

m<sup>3</sup>

Project: ATH-33-40.981  
 Date : 10/13/00  
 Calculated by: Ethan Schwartz

- \* = Figured by multiplying width of trench and area from culvert to elevation specified by ODOT.
- LP = Length column represents an area measured from left end of pipe to specified distance from centerline.  
 Volume figured by multiplying the given area by an ODOT specified depth
- RP = Length column represents an area measured from right end of pipe to specified distance from centerline.  
 Volume figured by multiplying the given area by an ODOT specified depth
- SR = Length column represents an area covering a specified station range.  
 Volume figured by multiplying area of range by an ODOT specified depth.



# OHIO DEPARTMENT OF TRANSPORTATION



1980 West Broad Street  
P. O. Box 899  
Columbus, Ohio 43216-0899



Reply to:  
Office of Materials Management  
1600 West Broad Street, Columbus, Ohio 43223-1298

September 25, 2000

Dodson-Stilson, Inc.  
6121 Huntley Road  
Columbus, Ohio 43229-1003

Attention: Mr. Todd Willis, P.E., P.S.  
Project Manager

Re: ATH-33-40.981, PID 18288  
Slope Review Submittal

Gentlemen:

We have completed our review of the Stage 2 submittal of the subject project. On August 25, 2000, we had finished reviewing the mainline cross-sections when it became apparent that our revised slopes would, in some cases, extend beyond the proposed right-of-way. At an August 28, 2000 meeting, the marked-up cross-sections were returned to DLZ. We received mainline cross-sections, corrected based on our recommendations and the resolution of the right-of-way issues, along with the side road plans and a final soils plan set on September 13 and 14, 2000. This correspondence covers our review of these submittals. Segments of this letter were intended to be included on the August 25 review, so some of the stated recommendations may already be completed.

On the cross-sections we have shown our estimated rock and soil cut-slopes for the 4-lane layout, and now the two-lane layout. Our main changes deal with the assumed thickness of the sandstone layers and the depth of overburden. We have also indicated areas where we feel granular material should be placed at the base of proposed fills. This is to allow compaction of the embankment material over wet, soft stream and ditch areas. We have shown changes throughout the mainline cross-sections, as well as on the TR 412, SR 681 and Old US 33 cross-sections. Stations 51+300 to 51+340 were missing from SR 681. Although it was listed on the Title Sheet, we assume there is no longer any Service Road 19.

There are several sections where coal seams will be present at subgrade or in the cut-slopes. When it occurred in the middle of a 2:1 cut, we kept it at 2:1. From Sta. 49+180 to 49+340, the coal was present immediately below sandstone. The intent is for the bench shown on these sections to be at the top of the coal layer.



Special benches are indicated on some of the cross-sections. Please calculate the quantities of excavation and embankment required and include them in the plans. Also, please include Note R102 in the General Notes. Some mainline benches have been added or changed, and CR 89 has some benches shown as well.

The end of the 30.980 section needs to correspond to the beginning of the 40.981 section. This includes a matching of the cut-slopes. The cut-slopes shown on our original marked-up set correspond to those shown at the end of the 30.980 section. Please coordinate the matching of the cut-slopes at the projects intersection.

It also came to our attention that the ditch fore-slopes on the 30.980 project were different than those shown for the 40.981 project, with a resulting approximately 4 m difference in the location of each ditch bottom in cut sections. This became evident when we tried to align the cut-slopes at the overlap stations between projects. We do not know if both fore-slopes are acceptable in general, but they need to at least match at the overlap stations on the plans. We leave it to the Consultants and District to determine the proper layout and we will address any changes at the resubmittals.

In several locations, the borings indicated wet, weak or unsuitable material near proposed subgrade or immediately beneath proposed embankment. This material should be removed from within the following stations:

41+940 - 41+980	Wet silty clays	0.5 m below existing groundline or below proposed subgrade, whichever is deeper
46+880 - 46+920	Weak A-4b silt	0.9 m below existing groundline or below proposed subgrade, whichever is deeper
48+180 - 48+220	Soft A-4b and clay	0.9 m below existing groundline or below proposed subgrade, whichever is deeper
48+640 - 48+700	Soft, loose clay, sand	1.2 m below existing groundline or below proposed subgrade, whichever is deeper

The width of this removal shall be 0.3 m beyond the edge of pavement in proposed cuts and the toe of the embankment in proposed fills. The excavated material shall be replaced with 203 Embankment.

The borings indicate 3-4 meters of soft silt and clay material present below the proposed embankment from Station 51+200 to 51+280 on SR 681. We recommend that the embankment be built up to the level of subgrade, then a waiting period of 30 days occur prior to final grading and paving.

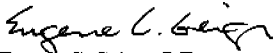
T. Willis  
September 25, 2000  
Page 3

There are several locations where conduits are being located under large fills, with soft soils as foundation material. Have any calculations been done to check for possible excessive or differential settlement? This could be a problem, especially at Culverts 24, 25, 28, 33, 40, and 43.

Please make the above recommended changes and resubmit the corrected plans. The revised plans should be quarter-size sheets, and both marked up sets enclosed should be returned as well so we have these sets to compare.

Please feel free to contact us should you have any questions.

Respectfully,



Eugene C. Geiger, P.E.  
Geotechnical Engineering Coordinator

Encl.

ECG/SAS

cc: S. Eldabaja - G. Collins - J. Townley - S. Sommers - Reading File - ~~1111~~



Dodson-Stilson, Inc.

A DIZ Company

ENGINEERS • ARCHITECTS • SCIENTISTS  
6121 Huntley Road  
COLUMBUS, OHIO 43229

(614) 848-4141  
FAX (614) 848-6712

# LETTER OF TRANSMITTAL

DATE	9-18-00	JOB NO.	9821-2016-00
ATTENTION	STEVE SOMMERS		
RE:	ATH-33-40.781		

TO ODOT

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

- Shop drawings
- Prints
- Plans
- Samples
- Specifications
- Copy of letter
- Change order
- \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
1		42	US 33 MAINLINE CROSS SECTIONS



THESE ARE TRANSMITTED as checked below:

- For approval
- For your use
- As requested
- For review and comment
- FOR BIDS DUE \_\_\_\_\_ 19\_\_\_\_\_
- Approved as submitted
- Approved as noted
- Returned for corrections
- \_\_\_\_\_
- Resubmit \_\_\_\_\_ copies for approval
- Submit \_\_\_\_\_ copies for distribution
- Return \_\_\_\_\_ corrected prints
- PRINTS RETURNED AFTER LOAN TO US

REMARKS \_\_\_\_\_

STEVE,

HERE ARE THE CROSS SECTION SHEETS  
THAT YOU REQUESTED AS PER OUR PHONE  
CONVERSATION. PLEASE FEEL FREE TO GIVE ME  
A CALL WITH ANY QUESTIONS AND/OR COMMENTS.

Brent Downing

COPY TO \_\_\_\_\_

SIGNED: Brent Downing



ENGINEERS • ARCHITECTS • SCIENTISTS  
 6121 Huntley Road  
 Columbus, Ohio 43229

(614) 848-4141  
 Fax (614) 848-6712

TO Ohio Department of Transportation  
Office of Materials Management  
1600 West Broad Street  
Columbus, Ohio 43223

LETTER OF TRANSMITTAL

DATE	9-14-00	JOB NO.	9821-3200.00
ATTENTION	Steve Sammers		
RE:	ATH-33-40, 981		

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

Shop drawings  Prints  Plans  Samples  Specifications

Copy of letter  Change order  \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
1	9-14-00		Soil Profile - Roadway Borings

THESE ARE TRANSMITTED as checked below:

- For approval  Approved as submitted  Resubmit \_\_\_\_\_ copies for approval
- For your use  Approved as noted  Submit \_\_\_\_\_ copies for distribution
- As requested  Returned for corrections  Return \_\_\_\_\_ corrected prints
- For review and comment  \_\_\_\_\_
- FOR BIDS DUE \_\_\_\_\_ 20 \_\_\_\_\_  PRINTS RETURNED AFTER LOAN TO US

REMARKS This set is complete except that sheets 111  
through 133 show the old cross sections.  
Sheets with the revised cross sections  
will be transmitted as soon as possible

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

COPY TO \_\_\_\_\_

SIGNED: Dorothy Adams



ENGINEERS • ARCHITECTS • SCIENTISTS

6121 Huntley Road  
Columbus, Ohio 43229

(614) 848-4141  
Fax (614) 848-6712

# LETTER OF TRANSMITTAL

DATE	9/13/00	JOB NO.
ATTENTION	STEVE SOMMER	
RE:	ATH-33-40.981	

TO 000T-Geotech

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

Shop drawings  Prints  Plans  Samples  Specifications

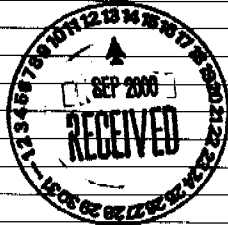
Copy of letter  Change order  \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
1 set			Revised ML x-sections per 000T-Geotech recommendations
1 set			ML & side road plans
1 set			side road x-sections for review

THESE ARE TRANSMITTED as checked below:

- For approval  Approved as submitted  Resubmit \_\_\_\_\_ copies for approval
- For your use  Approved as noted  Submit \_\_\_\_\_ copies for distribution
- As requested  Returned for corrections  Return \_\_\_\_\_ corrected prints
- For review and comment  \_\_\_\_\_
- FOR BIDS DUE \_\_\_\_\_ 20 \_\_\_\_\_  PRINTS RETURNED AFTER LOAN TO US

REMARKS \_\_\_\_\_



COPY TO \_\_\_\_\_

SIGNED: Pete VJ



# TRANSMITTAL LETTER

6121 HUNTLEY ROAD COLUMBUS, OH 43229-1003  
PHONE: (614) 888-0040 FAX: (614) 848-6712

DATE: October 16, 2000  
TO: Ohio Department of Transportation  
Office of Materials Management  
1600 West Broad Street  
Columbus, Ohio 43223-1298

PROJECT: ATH-33-40.981

ATTENTION: Eugene C. Geiger, P.E.

ACCOUNT NO:



### WE ARE SENDING THE FOLLOWING MATERIAL:

- |   |  |   |
|---|--|---|
| <input type="checkbox"/> Shop Drawings  | <input checked="" type="checkbox"/> Prints   | <input type="checkbox"/> Details        |
| <input type="checkbox"/> Samples        | <input type="checkbox"/> Tracings            | <input type="checkbox"/> Specifications |
| <input type="checkbox"/> Copy of Letter | <input type="checkbox"/> Request for Payment | <input type="checkbox"/> Change Order   |

### OTHER:

DATE	COPIES	DESCRIPTION
10-16-00	1	Revised Mainline Cross Sections per Slope Review Comments
10-16-00	1	Revised Side Road Cross Sections per Slope Review Comments
10-16-00	1	Revised Culvert Detail Sheets per Slope Review Comments
10-16-00	1	Stage I and II Marked Up Slope Review Sets

### THESE ARE FORWARDED:

- |  |   |   |
|--|---|---|
| <input checked="" type="checkbox"/> For your use | <input type="checkbox"/> For correction | <input type="checkbox"/> Reviewed as noted      |
| <input checked="" type="checkbox"/> For approval | <input type="checkbox"/> For your files | <input type="checkbox"/> For review and comment |

REMARKS: Here are the modified slopes for the ATH-33-40.981 project as per your comments. Please feel free to give me a call with any questions and/or comments.

CC: Doug Morgan, Saleh El-dabaja, File

BY: Todd Willis, P.E., P.S.

TITLE: Project Manager

## MEETING MINUTES

September 12, 2000

**Location:** Central Office, Room B Ground Floor  
**Subject:** Progress meeting for ATH/MEG-33-30.980/40.981

### Attendees:

Salch El-dabaja-ODOT	Jennifer Townley-ODOT	George Collins-ODOT
Debbie Fought-ODOT	Doug Morgan-ODOT	Bob Campbell-ODOT
Connie Gillum-ODOT	Ronald Riser-ODOT	Tom Davis-ODOT
Ken Dollison-ODOT	Dianna Hulsey-ODOT	Steve Sommers-ODOT
Noel Akala-ODOT	Wayne Pace-ODOT	Wayne Callahan-ODOT
Brent Downing-DLZ	Chuck Murphy-DLZ	Bud Naylor-DLZ
Pete Nix-DLZ	Johnny Ng-DLZ	Phil Schroeder-GF
Joseph Rikk Jr-GF	Tony Meacham-ME	Terry Winebrenner-ME
Ron Richards-ME	Erik Brokamp-ME	

### SOILS

- DLZ - We got approval from Gene Geiger's office to change the slope of the top layer of soil (at cut locations) from 4:1 to 3:1. Even so, we still have 2 places where the construction limits are beyond the current R/W lines. We also have a couple of areas where the construction limits are close to the current R/W lines. Right now, there are 2 properties affected by these changes in construction limits. Our R/W section is aware of the changes, however, we are not sure if those changes have been made.
- ODOT - Steve Sommers has indicated that he has not yet received all of the information that was due him for the review of the soils on the DLZ project. The District is concerned about the delay of the information on the additional borings. The final R/W is past due for completion, and any changes that may come from this review, will only further delay the final R/W plans.
- DLZ - All of the additional soil borings are complete, however, changes to the slopes based on these additional borings have not all been made to the cross-sections. We will have all of the information to Steve Sommers on Thursday, September 21.

### SCHEDULES

- DLZ - We will be submitting stage 3 for review on October 3, 2000. All of our bridges will be submitted for detail review by October 30, 2000. The first bridge will be submitted on September 26, 2000.

- GF/ME - Our Stage 3 submittal date has not yet been set by the District. The District has asked to delay our Stage 3 submittal date to provide ample time to review the DLZ section. Our Bridges will all be submitted for detail review on November 30, 2000.

### ROW

- ODOT - Skip Logan had a discussion with Saleh at one of latest TRAC meetings. He has been upset with the handling of his property appraisal. He has stated that the appraisal used comps that were far away from his property. He said he has several comps that are closer to his property. Someone needs to meet with Mr. Logan to determine if his comps are valid, and if so, use them to in the appraisal.
- GF/ME - Ron Richards will call Mr. Logan and set up a meeting to discuss his concerns.
- ODOT - Kevin O'Grady will be leaving SE Region and Wayne Callahan will be assuming Kevin's duties.
- DLZ - Our final ROW plans will be submitted to the District by September 29, 2000. We will also submit the plans to the designated Court Houses for checking.
- GF/ME - Our final ROW plans will be sent to the District at the beginning of October.

### UTILITIES

- ODOT - We need to make sure that we have all of the existing utility easements shown on the ROW plans. Texas Eastern has submitted a relocation cost and sketch of proposed work. Columbia Gas has an 18" line on the DLZ project, they may need some CADD files in order to expedite their relocation plan.
- DLZ - We have shown the Columbia Gas utility easement on our plans.
- GF/ME - We have discovered an AT&T easement for a fiber optic line that was not discovered during the court research. Phil Schroeder wants a copy of the Texas Eastern relocation sketch. We sent the City of Athens utility relocation plan to the District. We would like to have a meeting set up with the District and the City of Athens to discuss the plan.
- ODOT - Dianna Hulseley will contact the City of Athens and set up a meeting to discuss the utility relocation on Albany Road. The meeting will be set up for the week of September 25 thru September 29. (The Meeting has been set, and will be on Thursday, September 21 at 9:30 in Athens.)

### STRUCTURES



- ODOT - At the last meeting, there was a question about how much information the Consultants need to show on the plans for the reinforcing, since we are not supplying a reinforcing list with these projects. Do we need to show typical bar marks, or just the size and spacing of the reinforcing (along with a general shape of the bar)? Lloyd Welker and Jeff Crace have both said they would still like to have the typical bar marks shown.
- DLZ - Johnny Ng does not agree with the decision by L. Welker and Jeff Crace. He would like to have a meeting with Jeff Crace after this meeting.
- ODOT - We held a brief meeting with Jeff Crace, Jawdat Siddiqi, Tariq Mohammad, Doug Morgan, Johnny Ng and Joseph Rikk. As a result of this meeting, it was decided that the ODOT typical bar marks were not needed. The Consultants were instructed to provide a bar size, bar spacing and a general shape. The Consultants will have to make sure that reinforcing bars are detailed so that the fabricator will be able to detail the reinforcing bars correctly.

#### **OPEN DISCUSSION**

- ME/GF - All of our total take appraisals are completed, have been reviewed by SE Region and are in Ron Richards department. To date, Peter Chen's property is completed. We have a total of 21 total takes. At this time, 4 offers have been accepted and 3 other offers are pending.
- DLZ - We have 6 total take parcels on our project. At the present, one offer has been accepted. By next week, we will have made offers to all of the remaining total takes. It is Bud Naylor's opinion that the Story property at the corner of S.R. 681 and existing US33 will have to be appropriated. Bud has indicated that they are way apart on the price.

**The next meeting will be on Tuesday, October 10, 2000**

**@ 12:30 p.m. at Central Office on the first floor room 1C.**



# Dodson-Stilson, Inc.

A DIZ Company

ENGINEERS • ARCHITECTS • SCIENTISTS  
6121 Huntley Road  
COLUMBUS, OHIO 43229

(614) 848-4141  
FAX (614) 848-6712

# LETTER OF TRANSMITTAL

DATE	21 July 00	JOB NO.	9821-3200.00
ATTENTION	Scott Summers		
RE:	ATH - 33 - 40.981		
	Corrected Sheets		

TO Scott Summers  
ODOT - Central office  
Columbus, OH

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

- Shop drawings     Prints     Plans     Samples     Specifications  
 Copy of letter     Change order     \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
1			Corrected plan views for ATH-33-40.981

THESE ARE TRANSMITTED as checked below:

- For approval     Approved as submitted     Resubmit \_\_\_\_\_ copies for approval  
 For your use     Approved as noted     Submit \_\_\_\_\_ copies for distribution  
 As requested     Returned for corrections     Return \_\_\_\_\_ corrected prints  
 For review and comment     \_\_\_\_\_  
 FOR BIDS DUE \_\_\_\_\_ 19\_\_\_\_  PRINTS RETURNED AFTER LOAN TO US

REMARKS \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_  
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 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

COPY TO \_\_\_\_\_

SIGNED: Paul Rant



Dodson-Stilson, Inc.

A DIZ Company

ENGINEERS • ARCHITECTS • SCIENTISTS

6121 Huntley Road  
COLUMBUS, OHIO 43229

(614) 848-4141  
FAX (614) 848-6712

# LETTER OF TRANSMITTAL

DATE	11 July 02	JOB NO.	9821-3200.00
ATTENTION	GENE GEIGER, P.E.		
RE:	PARTIAL SUBMITTAL - 'DRAFT'		
	ATH-33-40.981		
	Soil PROFILES		
	MAINLINE		

TO ODOT  
1600 West Broad St., Room 2033  
Columbus, OH 43223

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

- Shop drawings     Prints     Plans     Samples     Specifications  
 Copy of letter     Change order     \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
1			LAST 5 Sheets of ATH-33-40.981 FOR REVIEW (Sht 48+270 to 49+890.950)

THESE ARE TRANSMITTED as checked below:

- For approval     Approved as submitted     Resubmit \_\_\_\_\_ copies for approval  
 For your use     Approved as noted     Submit \_\_\_\_\_ copies for distribution  
 As requested     Returned for corrections     Return \_\_\_\_\_ corrected prints  
 For review and comment     \_\_\_\_\_  
 FOR BIDS DUE \_\_\_\_\_ 19 \_\_\_\_\_     PRINTS RETURNED AFTER LOAN TO US

REMARKS GENE -  
ATTACHED ARE THE SHEETS DISCUSSED FOR THE PARTIAL DRAFT SUBMITTAL  
FOR ATHENS 33.

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COPY TO \_\_\_\_\_ SIGNED: Paul Panter

## Boring Locations

ATH-33-40.981

Mainline Borings (Drilled by Resource International)

Boring No.	Station	Offset (m)	Northing (m)	Easting (m)	Elevation (m)
B-80	39+577.378	24.9909L	135684.5000	637599.8000	273.40
B-81	39+777.419	0.0009R	135483.1000	637608.7000	229.60
B-82	40+027.356	0.0042R	135236.7000	637650.6000	269.90
B-83	40+227.359	0.3271R	135042.2000	637696.1000	247.10
B-84	40+337.259	0.2835R	134940.8000	637738.4000	231.90
B-85	40+447.258	50.2547R	134817.9000	637750.9000	256.30
B-86	40+467.197	0.3248R	134828.6000	637803.8000	245.90
B-87	40+567.188	0.0901R	134749.4000	637864.8000	246.00
B-88	40+707.294	80.0373R	134590.9000	637895.1000	271.90
B-89	40+747.261	0.0112L	134612.6000	637981.9000	263.40
B-90	40+777.194	27.9655L	134612.6000	638018.7000	259.00
B-91	41+025.279	0.0034R	134401.4000	638182.7000	261.00
B-92	41+227.215	0.2908L	134245.3000	638290.7000	263.30
B-93	41+317.162	49.6911R	134144.7000	638294.9000	227.50
B-94	41+327.197	50.3362L	134184.3000	638387.3000	229.20
B-95	41+527.134	29.6807R	133967.4000	638390.1000	293.10
B-96	41+687.280	50.3007L	133823.6000	638498.6000	256.90
B-97	41+817.004	49.9867R	133685.3000	638409.0000	291.00
B-98	41+926.954	0.0258L	133579.4000	638467.1000	270.20
B-99	42+086.973	4.9654L	133420.2000	638484.0000	256.00
B-100	42+223.044	19.5239L	133285.8000	638508.7000	281.00
B-101	42+347.984	1.9858R	133159.4000	638496.6000	270.00
B-102	42+471.921	41.9632L	133039.1000	638549.7000	274.00
B-103	42+647.018	0.0238R	132861.6000	638523.0000	259.10
B-104	42+826.994	0.0404L	132684.9000	638556.6000	252.70
B-105	42+927.063	49.9490L	132604.2000	638831.8000	261.40
B-106	43+087.068	0.0253R	132439.4000	638641.5000	255.80
B-107	43+227.000	50.0165L	132330.2000	638742.3000	274.90
B-108	43+227.081	49.9866R	132291.0000	638650.3000	269.20
B-109	43+227.041	0.0150L	132310.6000	638696.3000	272.30
B-110	43+427.061	0.0456R	132126.5000	638774.5000	263.90
B-111	43+577.027	0.0156R	131988.5000	638833.2000	267.80
B-112	43+746.977	0.0062R	131832.1000	638899.7000	232.70
B-113	43+907.068	50.0272R	131665.2000	638916.3000	231.50
B-114	43+907.027	0.0256R	131684.8000	638962.3000	214.40
B-114A	43+886.981	74.9973L	131732.8000	639023.5000	258.40
B-115	44+056.993	0.0044L	131546.8000	639021.0000	248.10
B-116	44+187.035	80.0098L	131450.6000	639127.1000	276.20
B-117	44+328.840	0.8185R	131296.3000	639126.6000	252.70
B-118	44+446.988	0.0247L	131187.9000	639173.8000	226.80
B-119	44+627.074	40.0243R	131006.5000	639207.2000	261.70
B-120	44+747.011	0.0455L	130911.8000	639291.0000	271.70
B-121	44+907.061	0.0261L	130764.5000	639353.6000	242.90
B-122	NOT DRILLED				
B-123	45+126.250	73.1596L	130591.3960	639506.6590	264.60
B-124	45+127.032	0.0002R	130562.0530	639439.6370	259.00
B-125	45+127.032	49.9998R	130542.4910	639393.6230	257.30

Boring Locations  
 ATH-33-40.981  
 Mainline Borings (Drilled by Resource International)

Boring No.	Station	Offset (m)	Northing (m)	Easting (m)	Elevation (m)
B-126	45+322.032	51.9999R	130362.2520	639468.0740	211.00
B-127	45+530.032	1.0002L	130191.5680	639598.2270	205.90
B-128	45+627.032	0.0004L	130101.9090	639635.2570	204.80
B-129	45+807.032	60.0005R	129912.7820	639650.4620	206.30
B-130	45+827.031	0.0004R	129917.8510	639713.5040	222.30
B-131	45+887.031	49.9999L	129882.1960	639782.9930	248.20
B-132	45+827.032	0.0004R	129825.8220	639752.6280	247.10
B-133	45+967.032	25.0009R	129779.2290	639745.2700	238.90
B-134	46+032.032	57.9997R	129706.5000	639740.3320	217.50
B-135	46+207.032	0.0004R	129568.1410	639862.1750	255.50
B-136	46+331.032	62.0000L	129478.2820	639987.7470	263.00
B-137	46+377.032	49.9996R	129392.1300	639862.6720	258.10
B-138	46+527.031	0.0002L	129273.6490	639987.3720	243.60
B-139	46+697.032	5.9998L	129119.5470	640059.4040	249.60
B-140	46+827.032	0.0004L	128997.5620	640104.7440	251.60
B-141	47+057.032	4.0006R	128784.3300	640191.0470	244.20
B-142	47+227.032	0.0004R	128629.4460	640261.2390	247.00
B-143	47+327.014	40.0127L	128553.6000	640337.1390	247.90
B-144	47+467.035	0.0745L	128411.1840	640360.8680	233.00
B-145	47+577.040	0.1238L	128314.7250	640413.7200	226.60
B-146	47+727.049	0.1899L	128187.5830	640493.2580	222.00
B-147	47+867.059	0.4689L	128073.9400	640574.9840	234.10
B-148	48+027.028	3.5998L	127947.5190	640673.0540	235.70
B-149	48+159.595	23.7839R	127824.8460	640730.2830	216.10
B-150	NOT DRILLED				
B-151	48+386.187	50.7115L	127687.6420	640925.3710	235.50
B-152	48+386.957	10.7187L	127663.1180	640893.7700	233.60
B-153	48+546.926	13.8823L	127536.7170	640991.8660	233.90
B-154	48+682.613	19.4411R	127407.9940	641046.1950	207.80
B-155	48+761.786	23.1327L	127369.9310	641127.6330	231.00
B-156	48+866.253	20.7475L	127283.9150	641188.0580	201.30
B-157	49+010.009	99.2765L	127196.3700	641333.7900	201.80
B-158	49+021.564	38.3466L	127157.1120	641285.6190	214.10
B-159	49+036.877	40.3164R	127107.5490	641222.6490	239.00
B-160	49+169.576	52.3357L	127022.5960	641360.8840	203.00
B-161	49+309.887	38.8738L	126879.5440	641390.9590	204.20

10/1/02

ADDITIONAL ATH-33 BORINGS  
 JOB NO. 9821-3200.00  
 METRIC

One's marked w/ 'X' can  
 be eliminated. Given by  
 phone to P. Nix: P. Painter  
 3/8/00. ECG.

m	<del>39</del> +635	37.5 Rt.	4.5	S.H.C/F >3m Fill 1:1	
m	<del>39</del> +635	63.5 Rt.	4.5	S.H.C/F >3m Fill @ E.E.	
m	<del>40</del> +030	58.0 Rt.	18.0	S.H.C. - use B-82 for x-sect.	
X L	40+300	50.5 Lt.	4.5	Additional Coverage	
H	<del>40</del> +488	37 Lt.	8.0	Transition + B-86 G.P. Hole - Missing Coverage	
H	<del>40</del> +604	51.5 Lt.	15	Transition + B-87 G.P. Hole - Missing Coverage	
m	<del>40</del> +825	CL	19	Transition + Coverage	
X L	40+990	CL	3	Transition	
H	<del>41</del> +230	CL	6	Redrill geoprobe hole B-92 - Missing Cut	
H	<del>41</del> +600	82 Rt.	27	Redrill geoprobe hole B-96	If only drill and boring drill Rt 82
H	<del>41</del> +600	CL	15	Transition - Coverage	
H	<del>41</del> +818	76 Rt.	19.5	B-97 Not deep enough - B/S max cut.	
H	<del>41</del> +900	42 Lt.	18	B-98 Not deep enough - max cut - transition coverage	
X L	42+190	CL	3	Transition	
X L	42+340	CL	3	Transition	
m	<del>42</del> +580	CL	13	Coverage (B-103 G.P.)	
H	<del>42</del> +600	Rt. 124	22	Cut Coverage	
✓ L	<del>43</del> +090	CL	3	Redrill B-106 G.P.	
X L	43+332	CL	3	Transition	
✓ L	<del>43</del> +620	CL	3	Transition	
H	43+905	Rt. 50	3	Redrill B-113 Geoprobe	pass. drill only one?
H	43+905	CL	3	Redrill B-114 Geoprobe + Questionable Location	
X L	44+105	CL	3	Transition	
✓ L	<del>44</del> +310	CL	5	Redrill B-117 Coverage	
X H	44+860	Rt. 48.5	12.5	S.H.C. >10m - B.S.	If only drill one boring drill Rt 28.
H	<del>44</del> +860	Rt. 28.5	17	S.H.C. >10m - U/S D.L.	

✓ L	<del>44</del> +920 <sup>a</sup>	Lt. 36	3	S.H.F. >10 - 1:1	
✓ L	<del>44</del> +920 <sup>b</sup>	Lt. 54	3	S.H.F. >10 - E.S.	
H	<del>45</del> +040	Lt. 62	18.5	S.H.C/F >3m Cut - (13.5) + Cover Transition	IF only drill one boring drill Rt 62
H	<del>45</del> +040 <sup>a</sup>	Lt. 28	10	S.H.C. >3m - U/S D.L.	
M-H	<del>45</del> +020 <sup>b</sup>	Rt. 36	3	S.H.F. >10m - 1:1	IF only drill one boring drill Rt 73
M-H	<del>45</del> +020 <sup>a</sup>	Rt. 73.5	3	S.H.F. >10m - E.S.	
H	<del>45</del> +400	Rt. 28	15.5	S.H. C/F - missing cut area coverage	
H-M	<del>45</del> +400 <sup>a</sup>	Lt. 30	8	S.H. C/F >10m Fill - 1:1	
M-H	<del>45</del> +400 <sup>b</sup>	Lt. 45.5	8	S.H. C/F >10m Fill - E.S.	
H	<del>46</del> +700	CL	10	Stability	
H	<del>46</del> +800	Rt. 27.5	7	S.H. C/F >3m Fill - 1:1	
H	<del>46</del> +860	CL	8	Transition + Coverage	
H	<del>45</del> +900 <sup>a</sup>	Lt. 48.5	23	S.H.C. >10m - B.S. + Coverage	IF only drill one boring dr. Lt. 48.5
H	<del>45</del> +900 <sup>b</sup>	Lt. 28	23	S.H.C. >10m - U/S D.L. + Coverage	
H	<del>46</del> +110	CL	11	Transition + Missed Coverage	
H	<del>46</del> +375	Rt. 50	25.1	Redrill B-137 (short) + Coverage	
H	<del>46</del> +520 <sup>a</sup>	Rt. 53	12.5	S.H. C/F - B.S. Missing Coverage	IF only drill one boring drill Rt 53
H	<del>46</del> +520 <sup>b</sup>	Rt. 28	11.6	S.H. C/F - U/S D.L. - Missing Coverage	
✓ L	<del>46</del> +558	CL	3	Transition	
M	<del>46</del> +570	Lt. 23	3	S.H. C/F - Fill 1:1	
X L	<del>46</del> +606	CL	3	Transition	
X L	<del>46</del> +700	Rt. 28	10	Cut - coverage - 1m below grade	
H	<del>46</del> +860	Rt. 52	13.5	S.H.C. - B.S. - Existing boring does not meet grade + transition	IF only drill one
H	<del>46</del> +860 <sup>a</sup>	Rt. 28	8	S.H.C. - W.S. - D.L. overburden thickness	
H	<del>47</del> +040	Lt. 40	13.3	Missing Top Cut Coverage	
H	<del>47</del> +040 <sup>a</sup>	CL	9	Auger Refusal then Core	Pickup new roadway
L-M	<del>47</del> +180	CL	9	Transition and Coverage	
H	<del>47</del> +300	Lt. 28	11	S.H.C. - U/S D.L. - Boring does not cover cut	
H	<del>47</del> +840	Lt. 65	16	S.H.C. - B.S. (CL boring missing top to grade)	

H	<del>48+240</del>	Lt. 45	12.5	B-148 Short - Coverage
L	48+100	CL	12	Transition + Coverage
M	<del>48+200</del>	Rt. 30	3	S.H. C/F >3m Fill - 1:1
L	<del>48+250</del>	CL	9.5	Transition + Coverage
H	48+260	28 Lt.	12.5	Transition and cut coverage (GP boring missing cut)
H	<del>48+300</del>	Lt. 10	13	B-151 does not meet grade coverage
H	<del>48+350</del>	Lt. 30	14.8	S.H.C. >10m - U/S D.L.
L	48+480	Lt. 30	3	S.H.C. @ ground surface - overburden thickness
L	<del>48+620</del>	CL	7	Transition + Coverage
H	<del>48+980</del>	Lt. 28.5	23	S.H.C. >10m - U/S D.L. (B-155 just above grade)
L	<del>48+1042</del>	CL	4	Transition and coverage (to Bridge borings)
H	<del>48+1080</del>	Rt. 114	38.5	S.H.C. >10m - B.S. - deep boring does not meet grade
X	49+020	Rt. 28	30	S.H.C. >10m - U/S D.L. - deep boring does not meet grade
H	<del>49+140</del>	Rt. 48.5	9	S.H.C. <10 - Max Cut
H	<del>49+260</del>	Rt. 104	50	S.H.C. >10m - B.S.
H	<del>49+260</del>	Rt. 31	19.5	S.H.C. >10m - U/S D.L.
X	49+260	Lt. 32	3	S.H.C. >10m - Confirm D/S Rx @ D.L.
H	<del>49+300</del>	Rt. 65	18.4	S.H.C. <3m - B.S.
H	49+400	Lt. 95	31.0	S.H.C. >10m - B.S.

If only drill one boring drill Rt. 114

RSECT\9821\320000\ADDL-AT.AEN





Dodson-Stilson, Inc.

A DIZ Company

ENGINEERS • ARCHITECTS • SCIENTISTS  
6121 Huntley Road  
COLUMBUS, OHIO 43229

(614) 848-4141  
FAX (614) 848-6712

# LETTER OF TRANSMITTAL

DATE	14 Feb 00	JOB NO.	9821-3200
ATTENTION	Gene Geiger		
RE.	ATH-33-40,981		
	Proposed Additional Roadway Borings		

TO ODOT  
1600 West Broad St., Rm 2033  
Columbus, OH 43223

WE ARE SENDING YOU  Attached  Under separate cover via \_\_\_\_\_ the following items:

- Shop drawings     Prints     Plans     Samples     Specifications  
 Copy of letter     Change order     \_\_\_\_\_

COPIES	DATE	NO.	DESCRIPTION
2			Additional Boring List w/ Prioritized listing.
1			3 sheets - Plan top for project.

THESE ARE TRANSMITTED as checked below:

- For approval     Approved as submitted     Resubmit \_\_\_\_\_ copies for approval  
 For your use     Approved as noted     Submit \_\_\_\_\_ copies for distribution  
 As requested     Returned for corrections     Return \_\_\_\_\_ corrected prints  
 For review and comment     \_\_\_\_\_  
 FOR BIDS DUE \_\_\_\_\_ 19 \_\_\_\_\_  PRINTS RETURNED AFTER LOAN TO US

REMARKS

*Mr Geiger -*

*Each boring location has a low, medium, high or in between classification. Also if you decide to only go with one boring instead of two for side hill cut we put our recommendation for which one. If you have questions please call. Thanks*

COPY TO \_\_\_\_\_

SIGNED: *Paul Brunt*

# OHIO DEPARTMENT OF TRANSPORTATION



1980 West Broad Street  
P. O. Box 899  
Columbus, Ohio 43216-0899



Reply to:  
Office of Materials Management  
1600 West Broad Street, Columbus, Ohio 43223-1298

September 27, 1999

Dodson-Stilson, Inc.  
6121 Huntley Road  
Columbus, OH 43229-1003

Attn: Mr. Todd D. Willis, P.E., P.S.  
Project Manager

Re: ATH-33-40.981, PID 18288  
Preliminary Slope Recommendations

Dear Mr. Willis:

Reference is made to your September 10, 1999 letter outlining the recommended preliminary cut and fill slope scheme for the subject project, the Soil Profile, and preliminary cross-sections. We are in general agreement with the scheme proposed. The scheme listed below indicates some minor changes to that which you have proposed. Note that for consistency, this letter with similar text is being sent to Gannett Fleming for use on the adjacent ATH-33-30.980 project. Also, please note that these recommendations are general in nature and some fine-tuning may be required upon a more detailed review of the preliminary plans and subsurface information.

## CUT SLOPES

Extend a 2:1 backslope to 1.5m above the back of the ditch and provide a 4.5m wide bench sloped at 10:1.

Soil. This includes are materials which were penetrated by the split-spoon sampler. Use a 2:1 slope where the thickness is less than approximately 1.5m. Use a 4:1 slope where thicker than approximately 1.5m. It may be necessary to transition over a couple of cross-sections between these two conditions, where they are present. Exceptions may be made to the 4:1 slopes if they would result in a thin sliver cut.

Shale and Mudstone. Use a 2:1 slope. Provide a 4.5m wide bench sloped at 10:1 at approximate 12.5m vertical intervals in the shale and/or mudstone. Likewise, provide the bench at the

Todd D. Willis, P.E., P.S.  
ATH-33-40.981  
September 27, 1999  
Page 2 of 2

interface where shale and/or mudstone is overlain by sandstone or siltstone.

Sandstone and Siltstone. Use a 1/2:1 slope where thicker than approximately 3m. Use a 2:1 slope where the thickness is less than approximately 3 m.

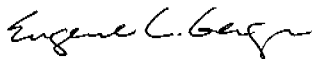
#### **FILL SLOPES**

Use 3:1 fill slopes for the ATH-33-30.980 project. For the ATH-33-40.981 project, use fill slopes of 2:1, except where the foundation is cohesive soils with N less than 4 or potentially unstable shales or mudstone.

Sidehill fills should be constructed utilizing Special Benching, generally on stable rock.

Please feel free to contact us should you have any questions.

Respectfully,



Eugene C. Geiger, P.E.  
Geotechnical Engineering Coordinator

c. S. Eldabaja, D. Briggs, Reading File, File



# Dodson-Stilson, Inc.

ENGINEERS • ARCHITECTS • SCIENTISTS

A DLZ Company

September 8, 1999

Ohio Department of Transportation  
Office of Materials Management  
1600 West Broad Street  
Columbus, Ohio 43223

Attn: Mr. Gene Geiger

Re: ATH-33-40.981  
Station 39+600 to 49+891  
Embankment Fill and Cut Slope Design  
DSI Job No. 9821-3200-00



Dear Mr. Geiger:

Enclosed are our proposed cross sections for the southern half of the Athens 33 project for your review. Also enclosed for your reference are the soil profiles and subsurface investigation reports for the entire project. It should be noted that the following recommendations are preliminary and need to be confirmed by field reconnaissance and stability analyses. The field reconnaissance is expected to be performed in late September or early October.

As the attached documentation explains, 2H:1V cut slopes are proposed in the mudstone and shale bedrock, whereas 1/2H:1V cut slopes are proposed for the sandstone and siltstone. Fall benches are included 1.5 meters above the drainage ditch and every 12.5 meters above the first bench. Using these guidelines, approximately 2.1 million cubic meters of sandstone and 2.1 million cubic meters of shale and mudstone will be removed in the cut sections.

The soil is generally less than one meter thick and frequently composed of an indurated clay or weathered shale, so a flatter cut in the soil is generally not used. In cases where the soil thickness exceeds 1.5 meters, a 4H:1V slope is used.

In cases where sandstone occurs above the shale, a 2H:1V slope is maintained in the sandstone as well due to stability concerns. Where this occurs, an additional bench is also shown at the base of the sandstone.

In most areas of the project south of Station 39+600, it is believed that 2H:1V fill embankment slopes can be utilized. However, in areas where the foundation consists of weak, highly compressible soils or rests on a slope of shale and/or mudstone, 3H:1V slopes are recommended. Using these guidelines approximately 3.7 million cubic meters of compacted fill will be required to construct the embankments.

# Dodson-Stilson, Inc.

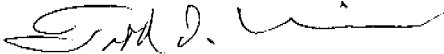
September 8, 1999

Page two

Please feel free to call myself or Pete Nix if you have any questions about this project.

Sincerely,

DODSON-STILSON, INC., a DLZ Company

A handwritten signature in black ink, appearing to read "Todd D. Willis". The signature is fluid and cursive, with a long horizontal stroke at the end.

Todd D. Willis, P.E., P.S.  
Project Manager

TDW/cb

Enclosures

cc: Mr. Doug Briggs, ODOT - District 10

PROJECT\9821\320000\GEIGER.DAA

# MEMO

**To:** Todd Willis  
**From:** Dorothy Adams  
**Subject:** Rock Cut Recommendations, ATH-33-40.891  
**Date:** May 21, 1999

We have reviewed the borings drilled for the roadway subsurface exploration and the rock cut recommendations presented by Resource International for the above mentioned project. Attached are our recommendations based on the borings information provided.

In general, there were two predominate types of rocks encountered: a very soft to medium hard shale or mudstone, much of which is also identified as "redbeds" in southeastern Ohio, and a medium hard sandstone. Minor amounts of medium hard siltstone and limestone were also encountered. Coal seams were not observed.

For the very soft to medium hard shale, we recommend that the rock be cut to a 2 horizontal to 1 vertical slope. If necessary, the shale may be cut as steep as 1:1. The medium hard sandstone and siltstone may be cut to a ½ horizontal to 1 vertical slope.

In addition, we present the following general recommendations:

1. The soil should be cut to a 2H:1V or flatter slope. Bedrock is generally shallow in Athens and Meigs Counties, however, the soil thickness at the boring locations ranged from 0.1 to 7.2 meters. If the soil layer above the rock is thicker than 3.0 meters, then a 3.0-meter wide bench should be cut along the base of the soil layer.
2. It is recommended that a 4.5-meter wide fall bench be constructed 1.5 meters above the roadside ditch. A 1:1 slope may be used between the ditch and the fall bench.
3. Where the rock cut height exceeds 12.5 meters, it is recommended that a 4.5-meter wide sidehill bench be constructed. Where the rock cut height exceeds 25.0 meters, a second sidehill bench will be necessary. All sidehill benches should be graded to allow for positive drainage along the slope to reduce flows over the exposed rock.

Details of the rock cut recommendations are in the attached table.

cc: Brent Downing

Section Information					Height of Cut (m)		Rock Cut Recommendations					Remarks
Begin Station	End Station	Side	Referenced Boring (s)	Soil Thickness (m)	Left	Right	Elev. of Cut (m)	Rock Type <sup>1</sup>	Cut Slope (Sb)	Ditch Slope <sup>2</sup>	No. Sidehill Benches	
39+500	39+620	Both	B-78	4.0	18-22	10-19	262 to soil	4	2H:1V	1:1	0-1	Cut bench at shale/sandstone interface at approx. el. 262 m
			B-79	1.3			ditch to 262	2	1/2H:1V			
			B-80	1.7								
39+620	39+675	Left	B-81	2.2	0-18	-	262 to soil	4	2H:1V	1:1	0-1	Cut bench at shale/sandstone interface at approx. el. 262 m
							ditch to 262	2	1/2H:1V			
39+835	39+936	Right	-		-	7-12	ditch to soil	4	2H:1V	1:1	0	
39+936	40+140	Both	B-82	2.1	3-6	5-17	255 to soil	4	2H:1V	1:1	0-1	
							ditch to 255	2	1/2H:1V			
40+140	40+180	Right	B-83	7.2	-	0-6	entire cut	Soil	2H:1V	1:1	0	
40+420	40+480	Right	B-85	0.6	-	0-6	ditch to soil	2	1/2H:1V	1:1	0	
			B-86	0.8								
40+560	40+700	Right	B-87	1.5	-	0-14	254 to soil	4	2H:1V	1:1	0-1	
							ditch to 254	2	1/2H:1V			
40+700	40+820	Both	B-88	1.8	3-5	6-18	256 to soil	4	2H:1V	1:1	0-1	
			B-89	1.2			ditch to 256	2	1/2H:1V			
			B-90	1.2								
40+900	41+040	Right	B-91	3.2	-	0-4	258 to daylight	soil	2H:1V	1:1	0	
							ditch to 258	2	1/2H:1V			
41+220	41+240	Both	B-92	1.7	2-5	0-3	262 to daylight	soil	2H:1V	1:1	0	
							ditch to 262	2	1/2H:1V			
41+240	41+257.162	Left	B-92	1.7	0-5	-	262 to daylight	soil	2H:1V	1:1	0	
							ditch to 262	2	1/2H:1V			
41+423	41+640	Both	B-95	1.8	3-19	8-30	entire cut	4	2H:1V	1:1	0-2	
41+660	41+740	Right	B-96	1.7	-	11-14	entire cut	4	2H:1V	1:1	0-1	
41+740	41+940	Both	B-97	0.9	4-20	0-25	entire cut	4	2H:1V	1:1	0-1	
			B-98	1.7								
41+940	41+980	Left	B-98	1.7	1-6	-	entire cut	soil	2H:1V	1:1	0	
42+180	42+300	Both	B-100	1.0	0-11	0-9	entire cut	4	2H:1V	1:1	0	
42+300	41+340	Left	B-101	4.0	0-7	-	entire cut	4	2H:1V	1:1	0	
42+400	42+540	Left	B-102	3.0	0-4	-	entire cut	4	2H:1V	1:1	0	
43+160	43+200	Left	B-106	0.1	2-6	-	entire cut	4	2H:1V	1:1	0	Medium hard sandstone may be encountered in ditch
			B-107	2.6								
			B-108	2.6								
			B-109	1.0								

1 - Rock types: 2 = sandstone or siltstone  
4 = mudstone or redbed shale

2 - Ditch slope of 2H:1V generally used

Section Information					Height of Cut (m)		Rock Cut Recommendations					Remarks
Begin Station	End Station	Side	Referenced Boring (s)	Soil Thickness (m)	Left	Right	Elev. of Cut (m)	Rock Type <sup>1</sup>	Cut Slope (Sb)	Ditch Slope <sup>2</sup>	No. Sidehill Benches	
43+200	43+340	Both	B-106 B-107 B-108 B-109	0.1 2.6 2.6 1.0	2-11	0-6	entire cut	4	2H:1V	1:1	0	Medium hard sandstone may be encountered in ditch
43+480	43+620	Both	B-111	1.2	0-4	2-7	ditch to soil	2	1/2H:1V	1:1	0	
43+620	43+640	Right	B-111 B-112	1.2 0.7	-	3-5	ditch to soil	2	1/2H:1V	1:1	0	
44+120	44+300	Both	B-116 B-117	1.0 2.3	4-19	0-9	259 to soil	4	2H:1V	1:1	0-1	
							ditch to 259	2	1/2H:1V			
44+580	44+660	Right	B-119	1.0	-	2-11	256 to soil	4	2H:1V	1:1	0	
							ditch to 256	2	1/2H:1V			
44+660	44+880	Both	B-120	1.6	5-19	11-22	252 to soil	4	2H:1V	1:1	0-1	
							ditch to 252	2	1/2H:1V			
44+880	44+920	Right	B-121	1.2	-	0-4	ditch to soil	2	1/2H:1V	1:1	0	
45+020	45+060	Left	-	-	6	-	244 to soil	4	2H:1V	1:1	0	
							ditch to 244	2	1/2H:1V			
45+060	45+260	Both	B-123 B-124 B-125	2.0 2.4 1.1	0-25	0-20	256 to soil	4	2H:1V	1:1	0-1	Cut slope above elevation 251 at 2H:1V
							251 to 256	2	1/2H:1V			
							244 to 251	4	2H:1V			
							ditch to 244	2	1/2H:1V			
45+380	45+460	Right	B-126 B-127	3.3 1.8	-	0-6	entire cut	4	2H:1V	1:1	0	
45+840	45+880	Left	B-130 B-131	3.0+ 1.0	6-19	-	ditch to soil	2	1/2H:1V	1:1	0-1	
45+880	46+000	Both	B-131 B-132	1.0 0.9	0-19	0-11	ditch to soil	2	1/2H:1V	1:1	0-1	
46+100	46+520	Both	B-135 B-136 B-137 B-138	0.9 1.0 1.8 1.0	4-28	2-22	entire cut	2,4	2H:1V	1:1	0-2	
46+520	46+620	Right	B-138	1.0	-	2-10	entire cut	4	2H:1V	1:1	0	
46+620	46+680	Both	B-139 B-140	1.7 1.0	0-10	2-10	243 to soil	2	1/2H:1V	1:1	0	Cut slope above elevation 243 at 2H:1V
							ditch to 243	4	2H:1V			
46+880	46+900	Right	B-140	1.0	-	2-6	entire cut	4	2H:1V	1:1	0	
46+960	47+000	Left	B-140 B-141	1.0 0.1	2-9	-	entire cut	4	2H:1V	1:1	0	

1 - Rock types: 2 = sandstone or siltstone  
4 = mudstone or redbed shale

2 - Ditch slope of 2H:1V generally used



Section Information					Height of Cut (m)		Rock Cut Recommendations					
Begin Station	End Station	Side	Referenced Boring (s)	Soil Thickness (m)	Left	Right	Elev. of Cut (m)	Rock Type <sup>1</sup>	Cut Slope (Sb)	Ditch Slope <sup>2</sup>	No. Sidehill Benches	Remarks
47+000	47+120	Both	B-141	0.1	0-10	0-6	ditch to soil	2	1/2H:1V	1:1	0	
47+160	47+180	Left	B-142	2.0	4-8	-	entire cut	4	2H:1V	1:1	0	
47+180	47+460	Both	B-142 B-143 B-144	2.0 2.6 3.0+	0-14	1-9	239 to soil ditch to 239	4 2	2:1 1/2H:1V	1:1	0-1	
47+680	47+720	Left	B-146	1.5	3	-	entire cut	4	2H:1V	1:1	0	
47+780	47+860	Left	B-147	2.0	4-9	-	232 to daylight ditch to 232	soil 2	2H:1V 1/2H:1V	1:1	0	
47+860	47+900	Both	B-147	2.0	3-9	0-3	232 to daylight ditch to 232	soil 2	2H:1V 1/2H:1V	1:1	0	
47+900	47+940	Left	B-147 B-148	2.0 1.7	3-9	-	232 to daylight ditch to 232	soil 2	2H:1V 1/2H:1V	1:1	0	
47+940	48+100	Both	B-148	1.7	0-10	3-7	ditch to soil	2	1/2H:1V	1:1	0	
48+180	48+340	Left	B-149 B-150	2.6 1.0	2-10	-	entire cut	4	2H:1V	1:1	0	
48+340	48+640	Both	B-151 B-152 B-153	3.7 1.9 2.6	1-14	0-13	ditch to soil	2	1/2H:1V	1:1	0-1	
48+720	48+800	Both	B-154 B-155	2.5 2.8	6-18	0-11	216 to soil ditch to 216	2 4	1/2H:1V 2H:1V	1:1	0-1	Cut slope above elevation 216 at 2H:1V
48+920	48+980	Right	B-156 B-157	3.7 2.9	-	0-31	216 to soil ditch to 216	2 4	1/2H:1V 2H:1V	1:1	0-2	Cut slope above elevation 216 at 2H:1V
48+980	49+280	Both	B-157 B-158 B-159 B-160	2.9 2.7 1.9 2.6	0-9	7-36	216 to soil ditch to 216	2 4	1/2H:1V 2H:1V	1:1	0-2	Cut slope above elevation 216 at 2H:1V
49+280	49+340	Right	B-161	2.6	-	9-25	entire cut	soil	2H:1V	1:1	0-1	
49+340	49+540	Both	-	-	-	-	entire cut	soil	2H:1V	1:1	0	
49+720	49+890.95	Left	-	-	-	-	entire cut	soil	2H:1V	1:1	0	

1 - Rock types: 2 = sandstone or siltstone  
4 = mudstone or redbed shale

2 - Ditch slope of 2H:1V generally used

**ESTIMATE OF QUANTITY OF SHALE AND  
MUDSTONE FROM CUT SECTIONS  
PROJECT ATH-33-40.981  
STATION 39+500 TO 49+891  
DODSON-STILSON, INC.**

Station		Location of Shale & Mudstone
From	To	
39+500	39+875	Above el. 262
39+835	40+160	Above el. 261
40+420	40+480	None
40+580	40+820	Above el. 256
40+920	41+120	None
41+200	41+251	Above el. 261
41+414	43+340	Everything
43+420	44+080	None
44+100	44+320	Above el. 260
44+580	45+000	Above el. 256
45+000	45+240	Between el. 244-248
45+300	45+440	Everything
45+840	46+240	None
46+260	46+600	Left of CL: above el 248 and below elevation 242
46+260	46+600	Right of CL: none
46+620	46+920	Below el. 244
46+960	47+100	None
47+160	47+440	Above el. 240
47+780	47+920	Above el. 232
47+940	48+100	None
48+340	48+800	None
48+927	49+120	Below el. 216
49+140	49+891	Everything

Any station not listed may be assumed to consist entirely  
of shale and/or mudstone.

# MEMO

**To:** Brent Downing  
**From:** Dorothy Adams *DA*  
**Subject:** ATH-33-40.981, Additional Cut and Fill Recommendations  
**Date:** September 3, 1999

Following are the revised cut and fill recommendations for the above-mentioned project, as we discussed on Thursday, September 2, 1999. Please disregard the previous memo on this subject dated August 24, 1999.

Embankment slopes of 3 horizontal to 1 vertical are recommended for the following fill sections:

40+160 to 40+300  
41+660 to 41+720  
41+940 to 42+200  
42+300 to 42+560  
45+800 to 45+860  
48+120 to 48+200  
48+640 to 48+700  
48+800 to 48+960

acceptable as shown with 2:1 and 3:1 slopes

It is expected that a bridge will be used instead of the embankment between Stations 45+600 and 45+800. All other fill sections may be designed with 2:1 slopes.

In the following cut sections, the soil should cut at a 4:1 slope:

41+900 to 41+940	top 1.7 meters, left slope only
44+700 to 44+760	top 1.8 meters, right slope only
45+080 to 45+200	top 2.4 meters, left slope only
47+240 to 47+360	top 2.6 meters, left slope only
47+780 to 47+900	top 2.0 meters, left slope only
48+360 to 48+400	top 2.1 meters, left slope only
48+740 to 48+780	top 2.1 meters, left slope only
48+980 to 49+100	top 1.9 meters

Assumptions for embankment slopes:

All slopes to be 2H:1V unless 1) more than one split-spoon sample had a standard penetration value of 4 or less in cohesive soil, OR 2) underlying bedrock consisted of potentially unstable shales and mudstones. Of particular concern are sections where the proposed embankment rests on an existing slope.

Assumptions for soil cuts:

Soil to be cut at a 4H:1V slope only where soil thickness exceeds 1.5 meters.

## CUT-SLOPE ALTERATIONS ATH-33-40.981

Limiting Stations	Maximum Increase (Meters)		Comments
	Left	Right	
39+600 to 39+657	12		Change most of 1/2:1 to 2:1 Change top 3 meters of 2:1 to 4:1
39+940 to 40+140	3	13	Change some of 1/2:1 to 2:1 Change top 4-6m of 2:1 to 4:1
40+440 to 40+480		20	Change upper 3 meters of 1/2:1 to 4:1
40+600 to 40+780		10	Change upper 3-4 meters of 2:1 to 3:1 or 4:1
41+240 to 41+500	7	18	Change upper 2-6 meters of 2:1 to 4:1
41+560 to 41+920	7	26	Change upper 3-8 meters of 2:1 to 3:1 or 4:1
42+200 to 42+320	7	8	Change 2:1 to 4:1
42+440 to 42+480	6		Change 2:1 to 3:1
43+200 to 43+300	6	3	Change upper 3 meters of 2:1 to 4:1
43+480 to 43+640		6	Change 1/2:1 to 2:1
44+100 to 44+300	10	8	Change upper 4-5 meters of 2:1 to 4:1
44+600 to 44+900	8	12	Change 1/2:1 to 2:1 Change upper 4 meters of 2:1 to 4:1 or 3:1
45+100 to 45+240	12		change upper 4-6 meters of 1/2:1 or 2:1 to 4:1
45+840 to 45+980	7	4	Change top 3 meters of 1/2:1 to 2:1
46+380 to 46+580	5	10	Change upper 3-5 meters of 2:1 to 4:1
46+700 to 46+740	4		Change upper 3 meters to 4:1 from 2:1
46+980 to 47+080	6	6	Change upper 4 meters of 1/2:1 to 2:1
47+160 to 47+240	11	3	change upper 3 meters of 2:1 to 4:1
47+280 to 47+320		3	Change upper 3 meters of 2:1 to 4:1
47+360 to 47+420	4	4	Change upper 3 meters of 2:1 to 4:1
47+940 to 48+100	16	9	Change upper 4 meters of 1/2:1 to 4:1
48+340 to 48+620	24	12	change some 1/2:1 to 2:1 Change some 1/2:1 to 4:1
48+720 to 48+760	9	4	change upper 3 meters of 1/2:1 to 4:1



8/16/99

Embk. - can work with red-bed, if proper construction  
inspection is done, soil is compacted  
at optimum (3:1), using rock core center

Define 'soil' + 'rock'; mix soil + red-bed to make  
embankment material

Need to check settlement of embankments

Dudman

Other area - soil a little better

Mostly 3:1, some 2:1 fills

(4 red bed) of sand cut ~~cut~~

to zone, core

9 4.7 mud fill

for 4 lanes

Compare cost between 3:1 and 2:1 gone

Cuts 1/2 sandstone, benches, 2:1 other spots

ATH-33-30.980/40.981

Mtg. 7/20/99

<u>NAME</u>	<u>ORG</u>	<u>PH.</u>
GENE GEIGER	ODOT-MAT MGT	275-1318
KIRK BEACH	"	275-1342
Steve Sommers	"	275-1356
Joe Rossie	Gannett Fleming	794 9424
Phil Schroeder	"	"
Pete Nix	DSI	848-4141
TODD WILLIS	"	"
BRENE DOWNING	DSI	848-4141
MITCH WEBER	GANNETT FLEMING	<del>794-9424</del>
DOUG FERIGUS	ODOT P-10	740-373-0212
DOUG MORHAN	"	"
STEVEN EUPARAJA	"	"

May 12, 1999

Ohio Department of Transportation - District 10  
Attention Mr. Douglas Briggs  
338 Muskingum Drive  
Marietta, Ohio 45750-0658

RE: Geotechnical Aspects of Embankment Fill and Cut Slope Design  
U.S. 33 Athens - Darwin Project (ATH - 33-30.980)  
Athens/Meigs County, Ohio

Mr. Douglas Briggs:

Presented herewith is an explanation and summary of the geotechnical review for Station 26+500 through Station 39+600 of the aforementioned project. This letter has been written per the request of The Ohio Department of Transportation (ODOT), District 10 to outline Gannett Fleming's concerns about the proposed highway alignment, as well as to suggest possible alternatives that may be available at this time for this project.

#### **Project Profile and Geological Conditions**

The section of highway assigned to Gannett Fleming begins at the intersection of Richland Avenue and Pomeroy Road (the present US 33) in Athens, Ohio and extends in a southerly direction for a distance of about 6 miles. At its present location, the proposed corridor traverses a region predominated by residual soils formed from Pennsylvanian aged sedimentary bedrock of the Marietta Plateau. Sandstone, siltstone, shale, clay shale/mudstone, along with occasional, deeper, thinly bedded coal seams dominate the bedrock profile along the proposed roadway corridor. Of particular concern are the soft shale and clay shale/mudstone units referred to as "Red beds", which appear dark maroon/purplish to red in color with occasional light gray marbling. These materials weather fairly quickly when exposed to form residual clays and silty clay soils of high plasticity and fairly low shear strength. As a result, many of the natural slopes formed in these materials exhibit signs of instability such as leaning trees (where vegetated) and toe bulges ("hummocky" appearance). More dramatic evidence of the innate instability in these materials can be found in constructed cut slopes and fill embankments along Interstate 77 and Interstate 70, as well as US 50 and US 33.

#### **Present Cut/Fill Slope Recommendations and Rational**

Based upon the proposed plans and profile elevations, substantial cuts and fills are planned for this project. Some of these cuts and fills are on the order of 30 to 40 meters in



magnitude. According to the information provided in the preliminary subsurface investigation by Resource International (Report No. W-7139), these cuts and fills will be constructed predominantly in, or with, these Red bed materials or residual soils derived from these Red beds. Approximately one-third of the laboratory index tests utilized to classify the residual clay and silty clay soils on this project corridor indicated materials of high plasticity (Liquid Limits higher than 50). An empirical correlation between these index tests and shear strengths yield conservative phi angle (angle of internal friction) estimates ranging from as low as 15 degrees to about 28 degrees. Utilizing a simplified infinite slope analysis, and choosing a phi angle ranging from 18 to 20 degrees, a preliminary slope ratio of 4 horizontal (H) to 1 vertical (V) has been chosen. If it is assumed that zero cohesion is available, this slope ratio would result in a factor of safety (F.S.) ranging from about 1.3 to about 1.45 with the aforementioned phi angles. Generally a F.S. of 1.5 is recommended for slope design. By choosing a 4H to 1V slope ratio for the weak soils in this project, there is the somewhat unconservative assumption that some cohesion is available over the design life of the project.

In some segments along the project alignment, relatively durable rock units comprised of massive sandstone, siltstone, isolated limestone layers and somewhat durable sandy to silty shale will be encountered in the road cuts. The combination of relatively high phi angles (greater than 40 to 45 degrees), unconfined shear strength values in excess of 25 to 30 Mpa, and slake durability values in excess of about 75 to 80 percent, accommodates steeper slope ratios in these materials. For this project, it is proposed to use 0.5H to 1V for the limestone, sandstone, and siltstone cuts and 1 H to 1 V slope ratios in cuts made in the less durable silty to sandy shale materials. Where these materials are encountered above the Red bed deposits in sufficient thicknesses to justify a change in a slope ratio, a three meter bench is recommended to reduce the possibility of undermining and subsequent rock falls in the harder more competent bedrock units.

Presently, it is proposed that the new highway will be a four lane divided roadway for slightly more than 2 miles south of the Richland Avenue intersection in Athens, then continue as a two-lane highway for the duration of its path in Athens County. Eventually, this alignment is to be upgraded to be a four-lane highway for the entire length of the project. Based upon how the project's first stage is to be constructed, it is estimated that approximately 6 million cubic meters of material has to be placed for embankment (fill) construction, while about 5.4 million cubic meters of material has to be excavated (cut). This would create a net shortage of fill material on the Gannett Fleming section of the project. Furthermore, it is roughly estimated that no more than about 500,000 cubic meters (less than 10 percent) of the cut material volume is comprised of what is considered durable rock. As a result, it is highly probable that the embankment fills for this project will be mostly comprised of materials derived from the Red bed deposits, which for reasons previously outlined, are not considered conducive to the steeper slopes often specified for standard ODOT construction (2H to 1V slope ratios). For this reason, it is proposed that 3H to 1V or flatter slope ratios be utilized for the embankment fills. This recommendation is rendered with the assumption that the material will be placed as a soil fill and that some granular material will be partially mixed in the fill as a result of the

excavation and placement activities that will effectively increase the long-term strength characteristics of the embankment soil.

### **Cut Slope Alternative - Additional Testing**

It should be noted that the aforementioned cut and fill slope ratio proposals are based primarily upon laboratory index test results with subsequent empirical shear strength correlations, typical experimental data obtained in somewhat similar materials, and, more importantly, experience with typical cut and fill slopes in the same geological formation. As a result, an effort is made to hopefully utilize a higher degree of conservatism. In the cut slope areas, the final slope will be comprised of materials that are naturally in place. The only way to justify the use of steeper slopes, which require higher shear strengths, would be to more accurately determine the soil parameters affecting long-term slope stability. To this end, specialized laboratory tests, such as three point triaxial tests, done specifically for this project would be necessary. Once the additional data is developed, a more comprehensive, computer-aided slope stability analysis should then be performed to examine different failure mechanisms. Ideally, the additional laboratory work and analysis will afford a higher degree of confidence in determining what can be done and what to expect in the cut slope areas.

### **Embankment Fill Slope Alternatives**

The fill slope areas of the project offer more flexibility for the designer, primarily because there is somewhat more control of the embankment composition. One option is to utilize durable rock buttresses at the embankment bases. This would provide a zone of material with higher shear strength characteristics at a critical location on the roadway fills, thus increasing the overall slope stability and possibly accommodating steeper slopes. Another option is to stabilize the Red bed materials prior to placement by mixing them with lime or cement and increase the shear strength of the proposed fill soil. A third option is to utilize a mechanically stabilized earth embankment in which a geogrid type material is placed in specified lifts as the embankment is constructed. This would result in a zone of reinforced earth with a higher effective shear strength characteristics that acts like a wall to intercept potential failure planes. One more option this letter will address is very similar to the rock toe buttress concept, which is the concept of building the embankments using an inner core and an outer shell. The inner core would consist of the residual clay and silty clay soils and the outer shell would consist of granular soils derived from durable rock. Any of these four options would have to be further analyzed for overall stability to determine how they should be designed and subsequent feasibility. They also have their drawbacks.

### **Rock Toe Buttress**

The rock toe buttresses will require a zoned type of construction and careful inspection as the embankments are built to make sure the buttresses are placed in the correct locations. Secondly, there will be a need to segregate and stockpile the durable rock material as it is excavated during construction. These inspection and segregation processes will increase

the construction time needed for the embankments. Thirdly the buttress concept is also limited by the availability of durable rock fill in the cut sections which may or may not be enough to complete all of the buttresses for the project. In the event that durable rock material must be imported to the project site, it should be noted that "local" rock quarries within a 20 to 25 mile radius of the project may not have the capacity to supply material in sufficient quantities for these buttresses. This is based upon an Ohio Mineralogical Industries Report of 1994, which indicated that the combined production from quarries in counties adjacent to the project site amounted to about 700,000 cubic meters in that particular year. Another consideration is the cost associated with hauling very large quantities of material over a potential 40 to 50 mile round trip. If a substantially greater amount of durable rock fill is needed than is available in the present road cuts, this buttress concept may be out of the question.

### **Soil Stabilization**

Chemical stabilization/improvement of the embankment soils would require preconstruction preparation to develop trial mixtures, perform subsequent lab tests, and establish the desired strength characteristics to determine the amount of additive needed for the soils. Once the project goes to construction the contractor has to develop a method to effectively and thoroughly mix the cementitious additive to large quantities of soil prior to placement, probably utilizing pug mills or other mixing equipment. The feasibility and success of the soil stabilization will depend on the amount and cost of the lime or cement additives and the efficiency with which the soil can be treated.

### **Soil Reinforcement**

The third option of geogrid stabilization will be affected by the material costs associated with the geogrid quantities required for the project. Then there is the need for the embankment construction to be thoroughly and carefully observed to insure that the soil reinforcement is appropriately installed. It is very likely that the geogrid placement operations and subsequent construction observation, coupled with the sizable embankments proposed for this project, will increase the construction time. Depending upon contractor competence, this increase in construction time could be substantial. The feasibility of this option will ultimately depend on the relative benefits from decreasing the mass embankment quantities (i.e. steeper slopes) versus the relative costs associated with the geogrid, increased construction time, and the possible need for other specified materials.

### **Inner Core/Outer Shell Construction**

The final option of inner core/outer shell construction (and any variations of such) is usually associated with embankment dam design, where the inner core performs as a relatively impervious barrier and the outer shell provides stability via gravity and increased shear resistance. With this type of construction, the outer shell is generally much larger than the inner core, requiring a substantial volume of durable rock material to complete

construction. For this reason, this method would have the same material limitation (limestone/dolomite and sandstone availability and haul distances) as the toe buttress. Another consideration is the construction inspection and increased construction time necessary to insure that the inner core and outer shell is properly placed. Finally, there is the issue of construction sequencing. The reason is that the inner core is generally constructed with steep (1H to 1V or steeper) side slopes with this method. In this project, the inner core will be constructed with residual soils that have been unstable at these slope ratios, meaning that the core must not be exposed for extended periods of time during construction. Otherwise, construction time embankment failures, which tend to involve deep failure zones and large soil masses, will probably occur. Ideally, the outer shell and inner core should be constructed simultaneously to essentially avoid core exposure.

### **Summary**

The earthwork quantities associated with this project are immense and, for the reasons stated in this letter, involve materials that are not compatible with typical ODOT embankment and cut slope construction methods. As a result, Gannett Fleming has proposed alternatives to these typical ODOT specifications that we feel should be used with this project to provide long term stability. These include 3H to 1V embankment fill slopes, 4H to 1V cut slopes in the residual Red bed materials, 1H to 1V cut slopes in the silty to fine sandy gray shale bedrock units, and ½H to 1V cut slopes in the sandstone, limestone, and siltstone bedrock units. Since there are substantial costs associated with implementing this set of recommendations, an attempt has been made to establish some possible alternatives that may allow more typical ODOT construction practices and decrease the amount of land acquisition required for this project. One alternative involves more comprehensive testing of the insitu materials, which may reveal the availability of higher long-term shear strengths. Other alternatives are associated with construction and include rock toe buttresses, soil improvement via lime or cement additives, soil reinforcement, and inner core/outer shell construction. Each of these construction alternatives, while potentially beneficial to the project, have their limitations and an attempt has been made to briefly discuss what they are.

If there are any questions regarding the content of this letter/report, please contact this office.

Sincerely,

**Gannett Fleming Engineers and Architects, P.C.**

John R. Kenny, P.E.  
Vice President  
Manager, Ohio Office

MDH

**AGENDA**  
**Cut Slope Design and Embankment Fill Considerations**  
**ATH-33 Athens to Darwin Final Design Project**  
**Station 29+500 to 39+600**

**1. Statement of Problem in Brief**

- 1.1. Natural materials in GF design section are principally composed of soft, highly fractured rock and weak residual clay soils.
  - 1.1.1. Problem strata are described as mottled red-gray or gray-purple Mudstone, Shale, Claystone, and Clay-Shale.
    - 1.1.1.1. Predominant in the northern and southern one-third of GF section.
    - 1.1.1.2. Cuts as deep as 40 meters are required. <sup>10% cuts</sup>
    - 1.1.1.3. Slickensides commonly identified. <sup>1-9m layers soil on top</sup>
    - 1.1.1.4. Core loss up to 100%, typically  $\approx$  25 to 35%.
    - 1.1.1.5. RQD often 0%, estimated average less than 30%.
    - 1.1.1.6. Soil Profile Report cites "mudstone and some shale ... deteriorated when exposed to water." and "rock condition was typically so poor that it was difficult to identify the transition from soil to rock".
  - 1.1.2. Residual soils from the weak 'redbeds' are also potentially problematic.
    - 1.1.2.1. Soil strata thickness varies from 1.0 to 5.0 meters,  $\approx$  2.0 meters.
    - 1.1.2.2. With respect to cut slopes, the residual soils will have shear strengths lower than the 'parent' rock material.
    - 1.1.2.3. Soils testing determined most of the rebed residual soils are high plasticity clays.
      - 1.1.2.3.1. About one-third of the samples tested from GF's section had Liquid Limits (LL)  $>$  50 %; however this soil represents more than one-half of the soil stratigraphy in GF's section. LL as high as 74% were determined.
    - 1.1.2.4. High LL correlate to high Compression Indices ( $C_c$ ) which indicate a high settlement potential.
    - 1.1.2.5. Soil Profile Report cites "natural moisture contents... are typically at or well above their corresponding Plastic Limits" (PL).

**1.2. Consequences to the Project**

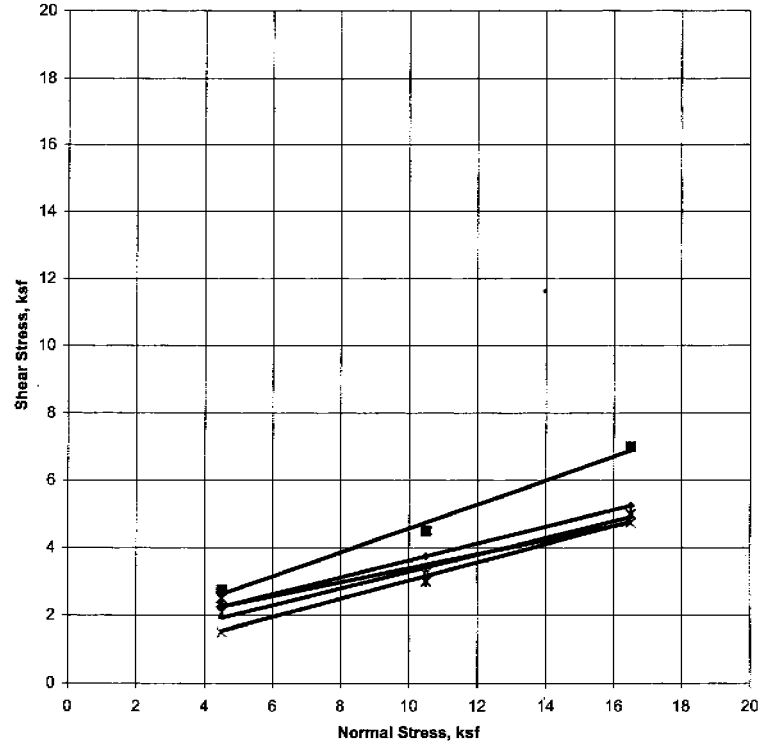
- 1.2.1. It is GF opinion that 'standard' 2:1 cut and embankment slopes are not appropriate for our section of the project where the rebeds predominate.
  - 1.2.1.1. GF's experience with rebeds in Ohio, West Virginia, and western Pennsylvania indicates that the shear strength of the rebed mudstone can be as low as 12° and generally falls between 16° - 19°. The results of over a dozen shear strength tests are represented on the attached summaries.

- 1.2.1.2. GF presented our approach to developing preliminary slope ratios for 4:1 slopes in the weak rock and residual soils of the redbeds.
- ✓ 1.2.1.2.1. GF recommended and the District has authorized additional drilling and testing for the 'roadway' portion of the project to determine shear strength, slaking and consolidation characteristics of the suspect strata.
- 1.2.2. The percentage of durable rock available for embankment construction is limited.
  - 1.2.2.1. Preliminary material balance calculations indicate that about 6 million m<sup>3</sup> are required for embankment construction. However, only 5.4 million m<sup>3</sup> are to be excavated. This indicates a slight imbalance and a net fill requirement for the section.
  - 1.2.2.2. The preliminary estimates of available durable sandstone, limestone, and siltstone from the required excavation is only 500,000 m<sup>3</sup> (<10% of the fill needed).
  - 1.2.2.3. As a result, it is probable that a significant amount of borrow will be need to construction standard 2:1 embankment or the redbed soil and rock strata will have to be used for embankment construction. Both alternatives have potentially undesirable consequences.
    - 1.2.2.3.1. If durable rock is brought in to construct the embankments, the construction cost will be significantly impacted due to import of durable material and waste of material from the required excavation.
- 1.2.3. The residual redbed soil potentially will cause handling and placement problems for construction of embankments.
  - 1.2.3.1. As stated in 1.1.2.5., the natural moisture content of the residual soils is at or significantly greater than the PL. The optimum moisture content to place clay soils is typically 2 to 4% greater than the soil's PL. This may require significant drying or admixture procedures in the field prior to placement of these soils in embankment construction.
  - 1.2.3.2. The high LL indicate a high potential for settlement of embankment construction of the residual soils.

The aforementioned problems are not insurmountable; however, the issues do require that ODOT and GF come to a consensus on the best approach to mitigate the problems. Several alternatives have been presented to ODOT in GF letter to Mr. Briggs in May 1999.

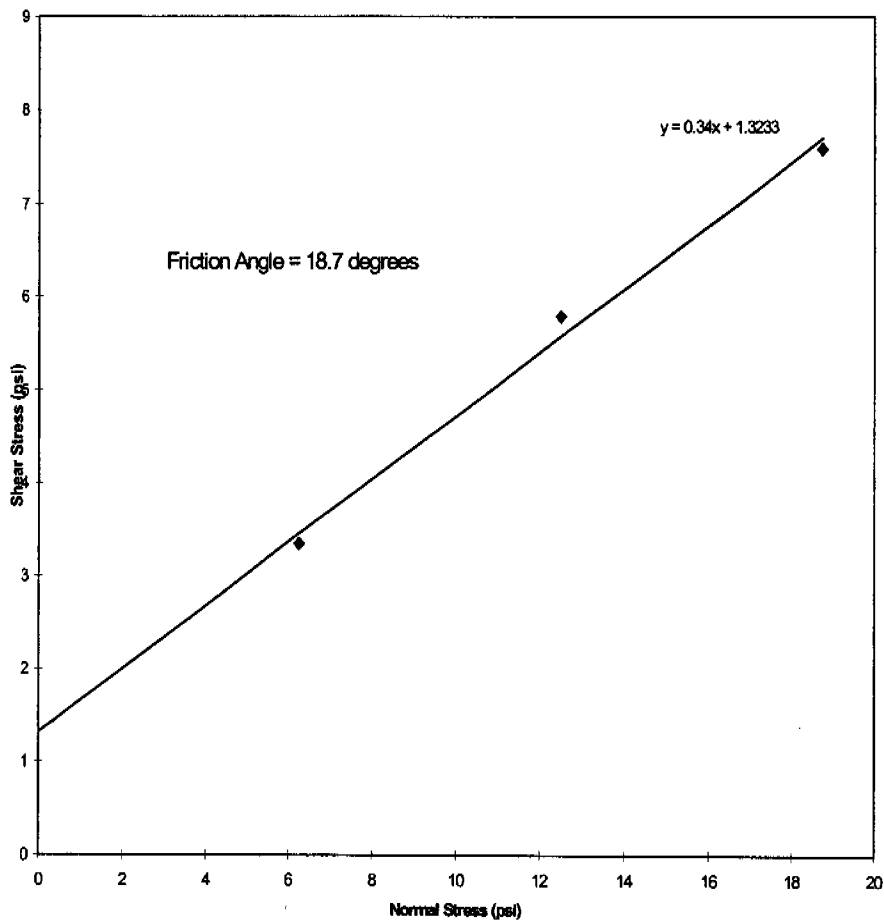
*Ramp area will need to be 2:1 for spacing between ramps + 11'*

Hughes River Dam Rock Core Shear Test Data





### Shear Stress vs. Normal Stress



# Daily Trip Report

## Office of Construction Administration

DTR01-7/03

NAME: Randall E. Morris  
State Construction Geotechnical Engineer  
& District 9 & 10 Advisory Construction Engineer

FIELD TRIP DATE: July 29, 2003

TOTAL TRAVEL TIME: 3 HRS

DISTRICT/PROJECT: 3 HRS

### PROJECT INFORMATION

DISTRICT: 10  
C-R-S: MEG-33-5,810  
PROJECT NUMBER: 425-01

TYPE: New Construction  
BID AMOUNT: 41 Million  
STATUS: 61% complete

### COMMENTS AND CONCERNS:

Chris Merklin, Bill Christensen and I met with the Project Engineer Jamie Hendershot to further assess the slide at stations of 36+520 to 36+620 left.

Chris reviewed his preliminary design with us. Some explanation was given about what to expect during the excavation.

The toe is to be benched into a black-grey shale and drained with drains from the top to bottom of the cut slope. Some of the survey data was not accurate enough to depict all of contours on the cross sections.

Many issues were discussed. Chris will make his recommendation on or before July 31, 2003 to Doug Morgan in Production. Doug will digitize the fix and calculate the quantities and give it to Construction.

This slide is the critical path of the project. It needs completed by the end of August to keep the project on schedule.

### PROJECT INFORMATION

DISTRICT: 10  
C-R-S: ATH/MEG-33-40.9  
PROJECT NUMBER: 246-01

TYPE: New Construction  
BID AMOUNT: \$ 33 Million  
STATUS: 67 % complete

### COMMENTS AND CONCERNS:

We briefly stopped by the project to look at the slide repair designed by Chris Merklin, which was in progress. The District changed the toe bench width to miss a gas line, were not taking compaction tests in the fill, had somewhat changed the benching slope configuration, did not encounter expected shale in the upper benches and added the rock in the toe but did not place the drainage in the benches.

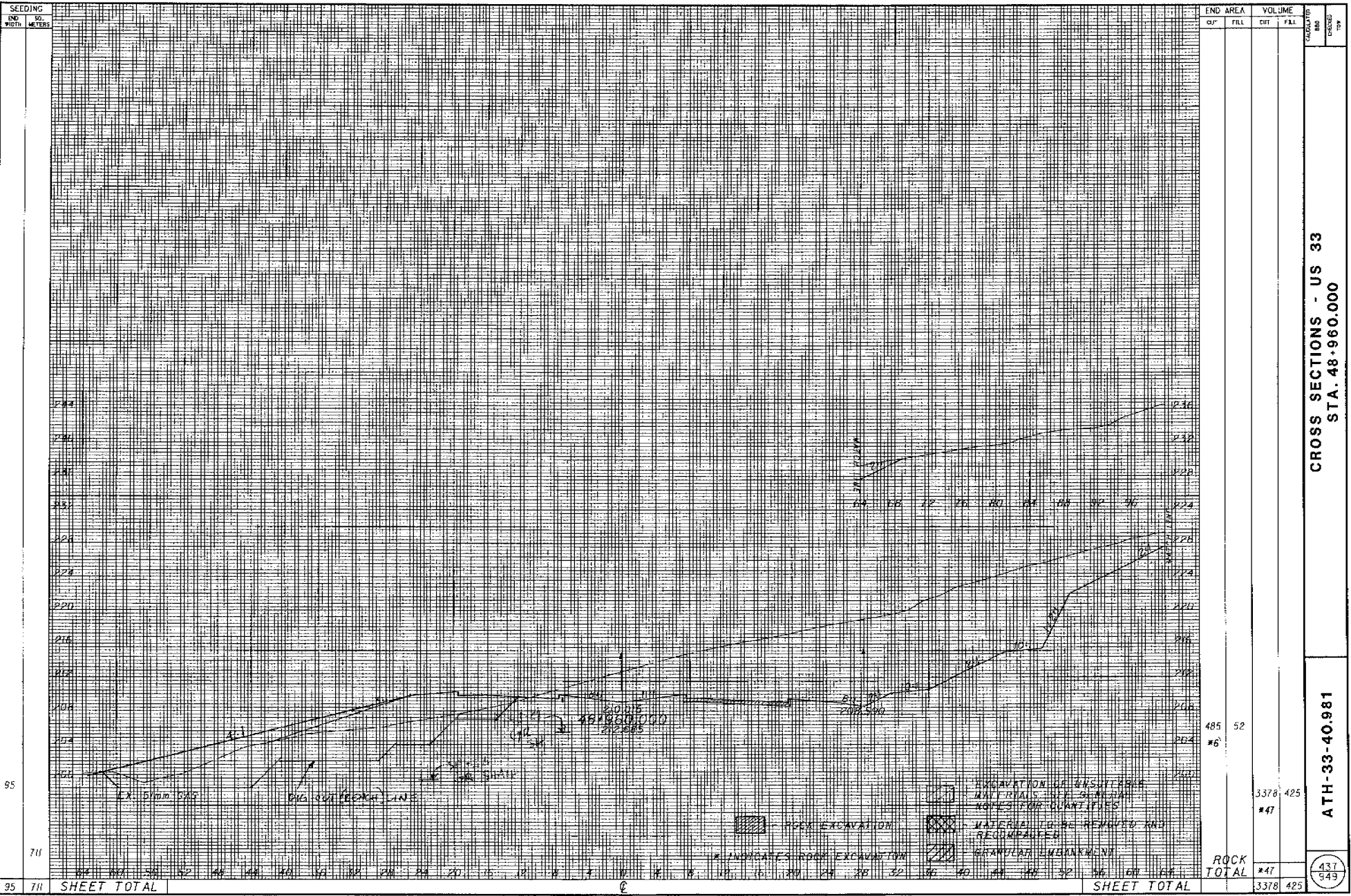
ATH-33-30.981 SLIDE REMEDIATION

ATH-33-40.981 SLIDE REMEDIATION

We visited two other projects, a project in District 10 under design and a project in District 5 under construction. The DCE requested us to evaluate a potential rock fall problem on an area to be blasted within the next few months.

cc: OCA Administrator  
DCE  
File

02/07/2011  
 M:\VPO\382\WIS-DC\Ave\wip\main\33sheet.dgn



CROSS SECTIONS - US 33  
 STA. 48+960.000

ATH-33-40.981

437  
 549

06/27/2014  
 08:00:37 PM  
 m:\p\011382\08-30\usa\main\me\_33xath.dgn



SEEDING		END AREA		VOLUME		CUMULATIVE	
END STATION	50 METERS	CUT	FILL	CUT	FILL	MEAS CUMULATIVE	TOY
160	1105						
SHEET TOTAL		370	55	2154	236		
		#6		#43			
		222	9	1445	43		
		#6		#33			
ROCK TOTAL						436	949
SHEET TOTAL				3599	279		

CROSS SECTIONS - US 33  
 STA. 48+944.818 TO STA. 48+952.098

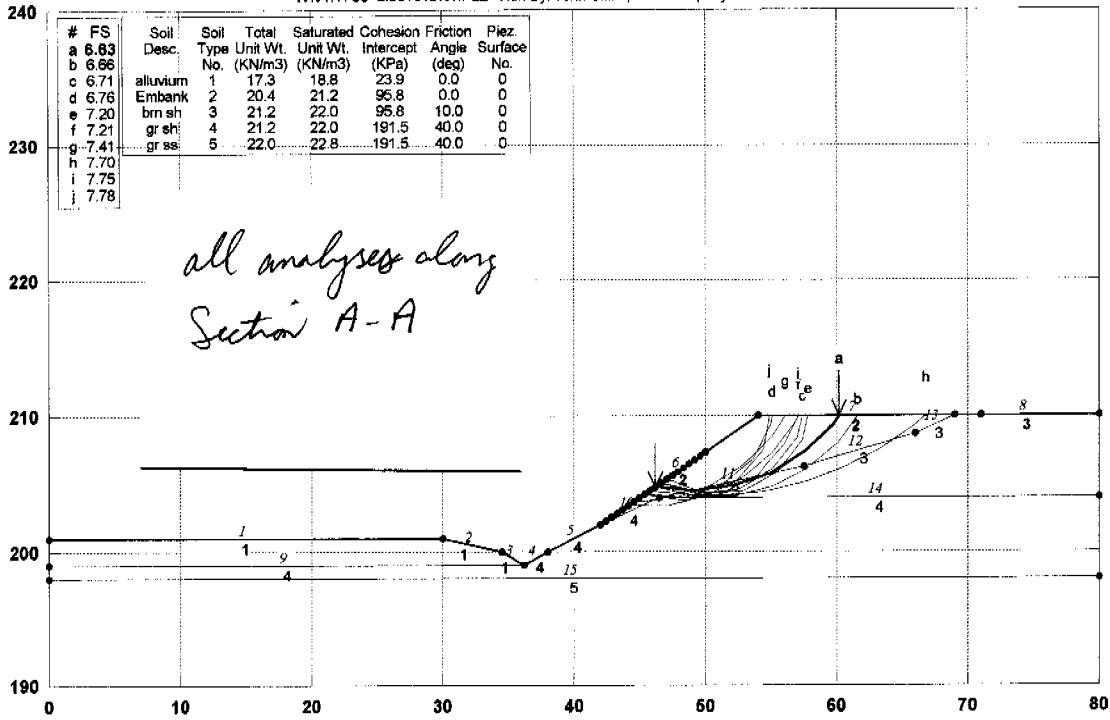
ATH-33-40.981

436  
 949

*original design*

**MEG-33-0581 circle, short term**

W:\ATH-33-2.981\RUN1.PL2 Run By: John Smith, XYZ Company 7/11/03 9:52AM



#	FS	Soil Desc.	Soil Type	Total Unit Wt. (KN/m3)	Saturated Unit Wt. (KN/m3)	Cohesion Intercept (KPa)	Friction Angle (deg)	Piez. Surface
a	6.83							
b	6.66							
c	6.71	alluvium	1	17.3	18.8	23.9	0.0	0
d	6.76	Embank	2	20.4	21.2	95.8	0.0	0
e	7.20	brn sh	3	21.2	22.0	95.8	10.0	0
f	7.21	gr sh	4	21.2	22.0	191.5	40.0	0
g	7.41	gr ss	5	22.0	22.8	191.5	40.0	0
h	7.70							
i	7.75							
j	7.78							

*all analyses along Section A-A*

PCSTABL5M/si FSmin=6.63

Safety Factors Are Calculated By The Modified Bishop Method

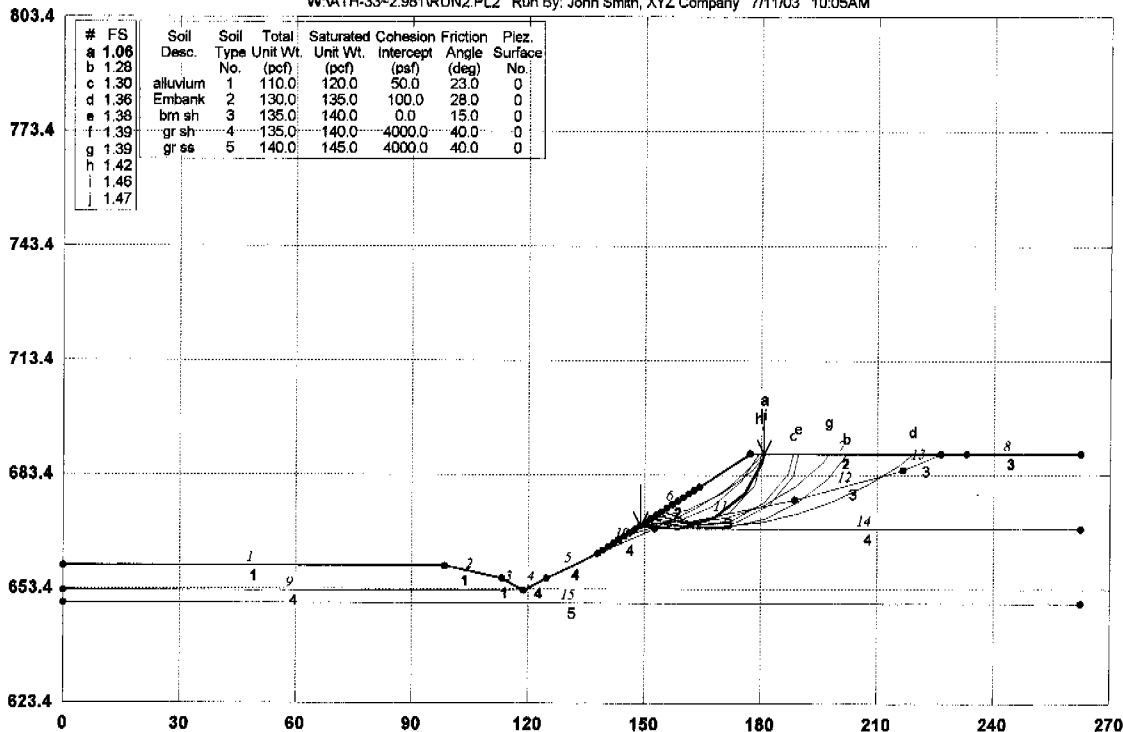
STED



*original design*

### MEG-33-0581 circle, long term

W:\ATH-33-2.981\RUN2.PL2 Run By: John Smith, XYZ Company 7/11/03 10:05AM



PCSTABL6M/si FSmin=1.06

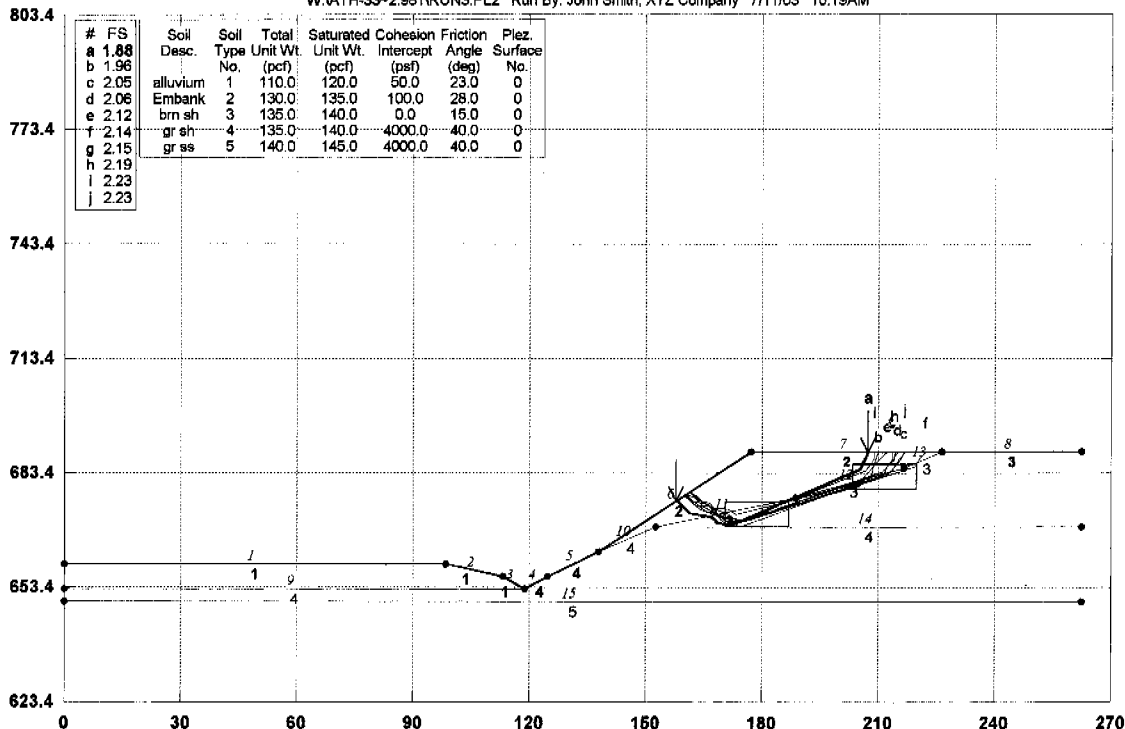
Safety Factors Are Calculated By The Modified Bishop Method



*original design*

**MEG-33-0581 block, long term**

W:\ATH-33-2.981\RUN3.PL2 Run By: John Smith, XYZ Company 7/11/03 10:19AM



PCSTABL5M/si FSmin=1.88

Safety Factors Are Calculated By The Modified Janbu Method

RTED

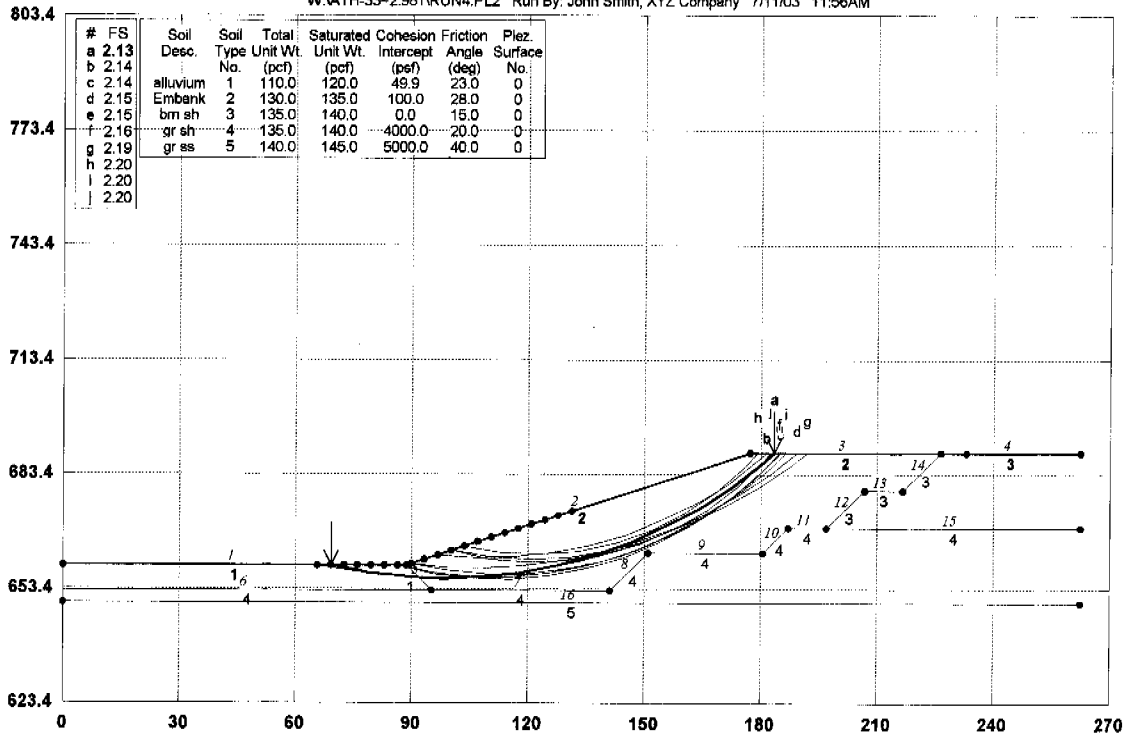




Fix

### MEG-33-0581 circle, long term with dig out 3:1

W:\ATH-33-2.981\RUN4.PL2 Run By: John Smith, XYZ Company 7/11/03 11:56AM



#	FS	Soil Desc.	Soil Type	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Piez. Surface No.
a	2.13							
b	2.14							
c	2.14	alluvium	1	110.0	120.0	49.9	23.0	0
d	2.15	Embank	2	130.0	135.0	100.0	28.0	0
e	2.15	bm sh	3	135.0	140.0	0.0	15.0	0
f	2.16	gr sh	4	135.0	140.0	4000.0	20.0	0
g	2.19	gr ss	5	140.0	145.0	5000.0	40.0	0
h	2.20							
i	2.20							
j	2.20							

PCSTABL5M/si FSmin=2.13

Safety Factors Are Calculated By The Modified Bishop Method

SITED



# FIELD DATA - SOIL LOG

Project Code

1378

Project Identification

MEG. 33-5.810

Station

000+00.20

Offset

Co., Rt., Br.No./Sec.No.

SEE

DRAWING

Order Code

01

Location No. B2

Over: SLIDE

Pier-Abut.

Started: 6-24-03

Equipment: R13

Completed: 6-24-03

Water Level: ~~5.0~~

Depth  
Feet  
Samples

Elevation

Description

Depth Feet	Samples	Elevation	Description
0			EXCAVATED GROUND
	④		BRN. CLAY W/STN. FRAGL 2.5'-4.0' B5-4-4 R 18" (1.2)
5			BRN. SHALEY CLAY
	⑤		5.0'-6.5' B18-18-17 R 18"
	⑥		BRN. SHALEY CLAY 7.5'-9.0' B8-11-23 R 18" (0.9)
10			BRN. SHALEY CLAY
	⑦		10.0'-11.5' B30-19-24 R 18"
	⑧		BRN. CLAY SHALE 12.5'-13.5' B50 R 8" 3.9
15			BRN. SHALEY CLAY
	⑨		15.0'-16.5' B23-31-48 R 18"
	⑩		GREY SHALEY CLAY 17.5'-18.5' B38-50 R 12" (0.64)
20			GREY SHALEY CLAY
	⑪		20.0'-21.0' B39-62 R 12" 6.4
			PIPE WET AT 23.0'
			GREY SHALEY CLAY
25	⑫		25.0'-26.0' B40-63 R 12" 7.92

Form 104  
DOT-1001-1/82

25		
30	⑬	WEAK GREY SHALE 30.0'-30.5' B78 R6" (0.25) PUT CORE BARREL IN AT 30.5'
35		GREY CLAY SHALE 30.5'-35.0' CORED REC. 4.5' 10.67
40		GREY CLAY SHALE TO GREY SANDY SHALE 35.0'-40.0' CORED REC. 5.0'
45		E.O.B. 40.0 (12.2m)
50		
55		
60		

Remarks: DRILLER CAREY

Party: SABO, PROCTOR

Chief of Party: McLEISH

# FIELD DATA - SOIL LOG

Project Code

Project Identification

1378

MEG. 32-5.810

Station

Offset

Co., Rt., Br.No./Sec.No.

0 + 0.6 1.6 1.1 1.1

SEE DRAWING

Order Code 01

Location No. B1

Over: SLIDE

Pier-Abut.

Started: 6-23-03

Equipment: B.S.P

Completed: 6-23-03

Water Level: 4.0 ft

BY HAND

Depth  
Feet  
Samples  
Elevation

LIV 01

Depth Feet	Samples	Description
0		0.5 TOPSOIL
1	①	BRN. SANDY CLAY
2		2.5-4.0 B.S.P. R18"
3		SCORE WET AT 4.0
4		BRN. SHALEY CLAY
5	②	5.0-6.5 B14-7-18 R16"
6	③	GREY SHALEY CLAY
7		7.5-8.0 B.S.P. R16"
10		DRILLED TO 10.0 TOP OF SANDSTONE 2.0
11		PUT SCORE BARREL IN AT 10.0
15		GOOD GREY SANDSTONE
16		10.0-15.0 SCORED REC. 5.0
20		GOOD GREY SANDSTONE
21		15.0-20.0 SCORED REC. 4.7
22		E.O.B. 20.0

25

30

35

40

45

50

55

60

Remarks: DALLER CARD

Party: CAREY, PROCTOR

Chief of Party: McLEISH



Site visit on  
6/5/03



Bridge @ E end of  
project (ATH-73-40.981)

ODOT 246-01

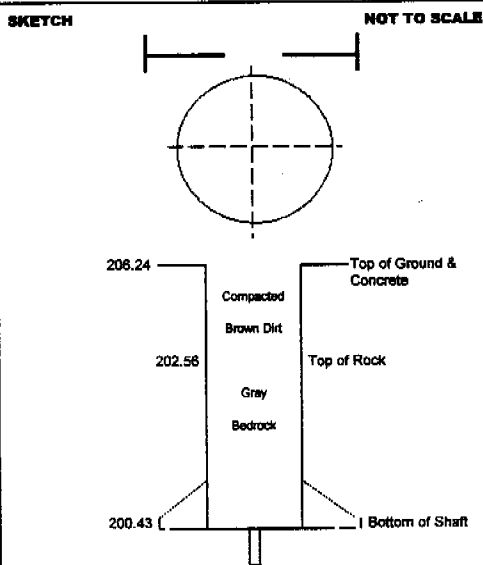
Cross section ~~436~~ 436  
48+940 (near abut)

- work w/ 48+952 cross-section
- mud flow-type failure

<b>Job Name:</b> O. D. O. T. 248-01	<b>Job Number:</b> F-6413-02
<b>General Contractor:</b> Smith & Johnson	<b>Architect:</b>
<b>Rig Number:</b> 12	<b>EWO #:</b> 0

DATES	START	FINISH	WEATHER	APPROX. TEMP. 65
<b>EXCAVATION:</b>	4/16/2002	4/16/2002	<b>DESCRIPTION :</b> Sunny	
<b>CONCRETING</b>	4/16/2002	4/16/2002		

SHAFT NO.:	PLAN	AS BUILT
29		
<b>Diameter Top of Shaft</b>	60	60
<b>Diameter Bottom of Shaft</b>	60	60
<b>Diameter of Bell</b>	N/A	N/A
<b>Elevation Top of Ground</b>	206.24	206.24
<b>Elevation Top of Concrete</b>	206.24	206.24
<b>Elevation Top of Rock</b>	203.26	202.56
<b>Elevation Bottom of Shaft</b>	201.16	200.43
<b>Cubic Yards of Concrete</b>	11.3	11.5



**SKETCH NOTES :**

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<b>CASEING USED</b>	N/A" ID X N/A" N/A"
	N/A" ID X N/A" N/A"
<b>TEST PROBE</b>	N/A" X N/A"

**Obstruction Removal :** N/A

**Time :** N/A

**Equip. Used :** N/A

**Excess Concrete :** N/A

**Description :** N/A

N/A

N/A

N/A

**REMARKS :** Drilled shaft, set full length cage after iron workers cut, poured concrete to grade.

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<b>APPROVED BY :</b>	<b>PREPARED BY :</b>
Russell Lee Smith & Johnson	Leonard N. Cluley Sr. <b>KOKER DRILLING COMPANY</b>

# INSPECTION RECORD FOR DRILLED SHAFTS

Project No.: 01-246 Bridge No.: Mag-33-05810 Structure File No.: 5300584  
 Drilling Contractor: Koker Drilling Co Project Engineer: Pete Stauffer / John Hook  
 Type & Model of Drilling Machinery: Watson 2500 Type of Bedrock: Med Hard Gray Slab  
 Max. Contiguous Torque (N-m) (Ft-Lb): 8,000 #16s Cost above the Bedrock (\$/LM) (\$/AF): 4,271.00  
 CROWD (Max. Cont. Downward Force) (kN) (Lbs): 38,000 #6s Cost in the Bedrock (\$/LM) (\$/AF): 16.70 ee  
 Cost of Concrete Pumping (\$/LM) (\$/LF): -

DRILLED SHAFT NUMBER	UNITS	#31	#30	#29	
<u>Forward Abutment</u>					

Date & Time of Drilling	Started	Date	—	4/16/02	—	—
		Time (Hr.)	7:00	7:50	8:45	
	Finished	Date	—	4/16/02	—	—
		Time (Hr.)	7:45	8:40	9:30	

Approximate Elevation of Top of Overburden	(m) (Ft)	206.24	—	—	—
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REF 274

Length of Drilled Shafts above the Bedrock Socket	Elev. Top of Case Through <del>Overburden</del>	(mm) (Ft)	206.24	—	—	
	Through Overburden	(mm) (Ft)	2.74	3.53	3.68	
	Pay Length	(mm) (Ft)	2.74	3.53	3.68	(9.95) Ref# 274

Obstructions Encountered	Number	—	—	—	—
	Size (mm) (in)	—	—	—	—
	Time of Removal (Hr.)	—	—	—	—

REF 275

Length of Drilled Shafts in the Bedrock Socket	Elev. Top of Socket (m) (Ft)	203.50	202.71	202.56	
	Elev. Bottom of Socket (m) (Ft)	201.37	200.58	200.43	
	Length of Socket (mm) (Ft)	2.13	2.13	2.13	(6.39) Ref# 275

Steel Casing	Casing Thickness (mm) (in)	—	—	—	—
	Casing Left in Place (mm) (Ft)	—	—	—	—

Reinforcing Steel	Vertical	Bar Size Number	—	36mm	—	—
		No. of Bars	—	20	—	—
	Spiral	Bar Size Number	—	13mm	—	—
		Pitch (mm) (in)	—	115mm	—	—

Concrete	Slump (mm) (in)	140	—	—	—
	Cylinder Strength f <sub>c</sub> (MPa) (psi)	34.3 / 32.8	—	—	—
	Air Temperature (C) (F)	30°C	—	—	—
	Time to Place Conc. (Hr.)	OK	—	—	—
	Quantity (CM) (CY)	58.106	—	—	—

Tolerances	Deviations from Plumb		N-S (mm) (in)	OK	—	—
			E-W (mm) (in)	OK	—	—
			Deviations of Column Top Center from Plan Loc. Horizontal (mm) (in)	2"	1 1/2"	1/4"
			1 1/2"	2 1/4"	1"	ACROSS Abutment

Shaft Diameter (Bedrock / Overburden)	(mm) (in)	1525	—	—	—
Constructed Diameter (Bedrock / Overburden)	(mm) (in)	1525	—	—	—

**PROJECT ENGINEER'S COMMENTS:** On BACK, please comment on location and extent of cavities, procedures for controlling water, unexpected subsurface conditions and suggestions on improving the plans.

State of Ohio  
Department of Transportation  
INSPECTOR'S DAILY REPORT

Lo-55  
Hi-88  
Sunny & clear

246 Co./Rt./Sec. Alt-33-40.981 Diary Date 4/16/02 Tues I.D.R. # \_\_\_\_\_

Check if additional Sheet Required. \_\_\_\_\_

Page \_\_\_\_\_ of \_\_\_\_\_

CONTRACTOR'S HOURS WORKED AND NUMBER OF EMPLOYEES

Contractor	Supt./Foreman	Hours			Number of Employees			
		From	To	Total	Super.	Skilled	Labor	Other
Kokee Drilling	Len Cluley	7:00	6:30	11	1	2		

Description of Work: Drilled shafts and poured concrete to elevations and alignment provided by S&J Co.

Contractor	Supt./Foreman	Hours			Number of Employees			
		From	To	Total	Super.	Skilled	Labor	Other
S&J	Russell Lee	7:00	6:30	11	2	1	1	

Description of Work: LAYOUT placement of drilled shafts and directed IRON workers on building and cutting cages to rebar lengths. Rod shoulder's assisted Rod in AM hours in preparation for Pile #2.

Contractor	Supt./Foreman	Hours			Number of Employees			
		From	To	Total	Super.	Skilled	Labor	Other

Description of Work: \_\_\_\_\_

CONTRACTOR'S EQUIPMENT

Contractor	Equipment Nbr	Equipment Type	Idle (Yes/No)
Kokee Drilling Co		Watson Rotary Drilling	
	266	Foreman Pickup Truck	
		Utility Truck	
S&J		Foreman Pickup Truck	

EMPLOYEE DATA

Employee	Nbr. Hours Worked	Work Code	License Number
John Hook	12	51-91	1072

\*OT Explanation: \_\_\_\_\_







Chris Merklin

06/20/03 07:33 AM

To: William Stanforth/Construction/D10/ODOT@ODOT, Charles  
Mansfield/Construction/D10/ODOT@ODOT  
cc: Hank Hager/Geotech/CEN/ODOT@ODOT, Jamie  
Hendershot/Construction/D10/ODOT@ODOT, Randy  
Morris/Construction/CEN/ODOT@ODOT  
Subject: MEG-33-5.810 slide

**CORRECTION: Please stake a boring at 48+960, 15 meters left, rather than 950, as stated below.**

Christopher Merklin, P.E.  
ODOT - Office of Geotechnical Engineering  
1600 West Broad Street  
Columbus, Ohio 43223  
ph: 614-275-1361 fax: 614-275-1354  
email: chris.merklin@dot.state.oh.us

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Pete/Charlie:

Do you have any electronic pictures of the slide at 48+940? If so, please send them.

Can you please survey the existing ground and create a topographic plan view? Also, I will need cross-sections at 48+952.098, 48+960, 48+970 (all showing existing groundline).

We would like to drill two borings at this site. Since you seem to be in more of a hurry here than the other slide (at 36+570), we will drill this one starting Monday. Please stake borings at stations 48+950, 50 meters left and 15 meters left. The boring at 50 meters left is an ideal location. According to the plans, the boring falls between the stream and a gas line, about 5 meters from both. If this location doesn't work, my second choice is at 48+935, 60 meters left, on the other side of the gas lines (there appears to be 4 gas lines in this area!).

Please provide the drillers with the necessary access to these boring locations, including dozer access. Also, please provide the drillers with the station, offset, and elevation of the boring locations before they leave the site.

If you have any questions, please call.

Christopher Merklin, P.E.  
ODOT - Office of Geotechnical Engineering  
1600 West Broad Street  
Columbus, Ohio 43223  
ph: 614-275-1361 fax: 614-275-1354  
email: chris.merklin@dot.state.oh.us

## inter-office communication

to: Saleh Edldabaja date: 8/26/03  
from: Mark Stouffer, P.E., Operations  
subject: Report of Landslide Investigation - MEG-33-5.810

Transmitted herewith are the results of the drilling, sampling and testing phases of the landslide investigation made for the subject project.

The enclosed design file consists of two logs of borings which are to be attached to the plans.

If there are any questions, please call Mark Stouffer at 275-1307.

If any design assistance is needed, please call Chris Merklin at 275-1361.

Jack

cc: C. Merklin  
R. Morris

LOG OF BORING

Date Started 6/23/03 Sampler: Type 34.93 mm Dia. Water Elev. 199.98m Project Identification: MEIGS  
Date completed 6/23/03 Approx. MEG-33-5,810 (3.6mi.)  
Boring No. B-1 Station & Offset 100+000, 42.1m LT. OF BL Surface Elev. 201.20 m LANDSLIDE INVESTIGATION

Elev.	Depth	Std. Pen./ ROD	Rec. ■	Loss m	Description	Sample No.	Physical Characteristics							ODOT Class		
							% Agg	% C.S.	% F.S.	% Silt	% Clay	L.L.	P.I.		W.C.	
201.20	0				0.15m TOPSOIL										VISUAL	
201.05		AUGERED														
200.44	1.0	2/3/3			BROWN SANDY SILT (REC. 0.30m)	1	0	20	50	17	13	NP	NP	19	A-3a	
199.68					TOP OF ROCK											
198.91	2.0	14/7/19			BROWN SILTY CLAY (REC. 0.39m)	2	0	9	7	26	58	35	15	12	A-6a	
198.46		50(0.15)			GRAY BROKEN AND JOINTED CLAY-SHALE (REC. 0.15m)	3	-	-	-	-	-	-	-	-	VISUAL	
198.46	3.0	AUGERED			SANDSTONE (DRILLER'S DESCRIPTION)										VISUAL	
198.16																
	4.0		1.52	0.00	SANDSTONE, GRAY, LIGHT GRAY AT THE BOTTOM, SLIGHTLY MICACEOUS, FIRM, FINE-GRAINED, THIN TO MEDIUM-BEDDED, JOINTED IN THE UPPER PORTION, WITH SCATTERED THICK BROKEN AND JOINTED SEAMS BELOW; INTERBEDDED WITH A THICK GRAY HIGHLY WEATHERED SHALE SEAM IN THE LOWER PORTION. CORE LOSS 3 1/2 R.O.D. IS 37											
	5.0	37														
			1.43	0.09												
195.10	6.0															

↑  
BOTTOM OF BORING

Particle Sizes: Agg= >2.00mm, Coarse Sand= 2.00-0.42mm, Fine Sand= 0.42-0.074mm, Silt= 0.074-0.005mm, Clay= <0.005mm