

STATE OF OHIO
 DEPARTMENT OF TRANSPORTATION
MOT-75-14.60
 PRESTRESSED CONCRETE OPTION
 CITY OF DAYTON
 MONTGOMERY COUNTY

PROJECT DESCRIPTION

THE PROJECT CONSISTS OF COMPLETE REPLACEMENT OF THE EXISTING IR-75 STRUCTURE OVER THE GREAT MIAMI RIVER WITH A NEW 3 SPAN CONTINUOUS AND COMPOSITE HYBRID STEEL PLATE GIRDER STRUCTURE, RAMP MODIFICATIONS IN ALL FOUR QUADRANTS OF THE IR-75 AND STANLEY AVENUE INTERCHANGE, AND SEVERAL RAMP CLOSURES WITHIN THE PROJECT LIMITS.

EARTH DISTURBED AREAS	
PROJECT EARTH DISTURBED AREA	11.25 ACRES
ESTIMATED CONTRACTOR EARTH DISTURBED AREA	0.60 ACRES
NOI EARTH DISTURBED AREA	11.85 ACRES

LIMITED ACCESS

THIS IMPROVEMENT IS ESPECIALLY DESIGNED FOR THROUGH TRAFFIC AND HAS BEEN DECLARED A LIMITED ACCESS HIGHWAY OR FREEWAY BY ACTION OF THE DIRECTOR IN ACCORDANCE WITH THE PROVISIONS OF SECTION 5511.02 OF THE OHIO REVISED CODE.

2005 SPECIFICATIONS

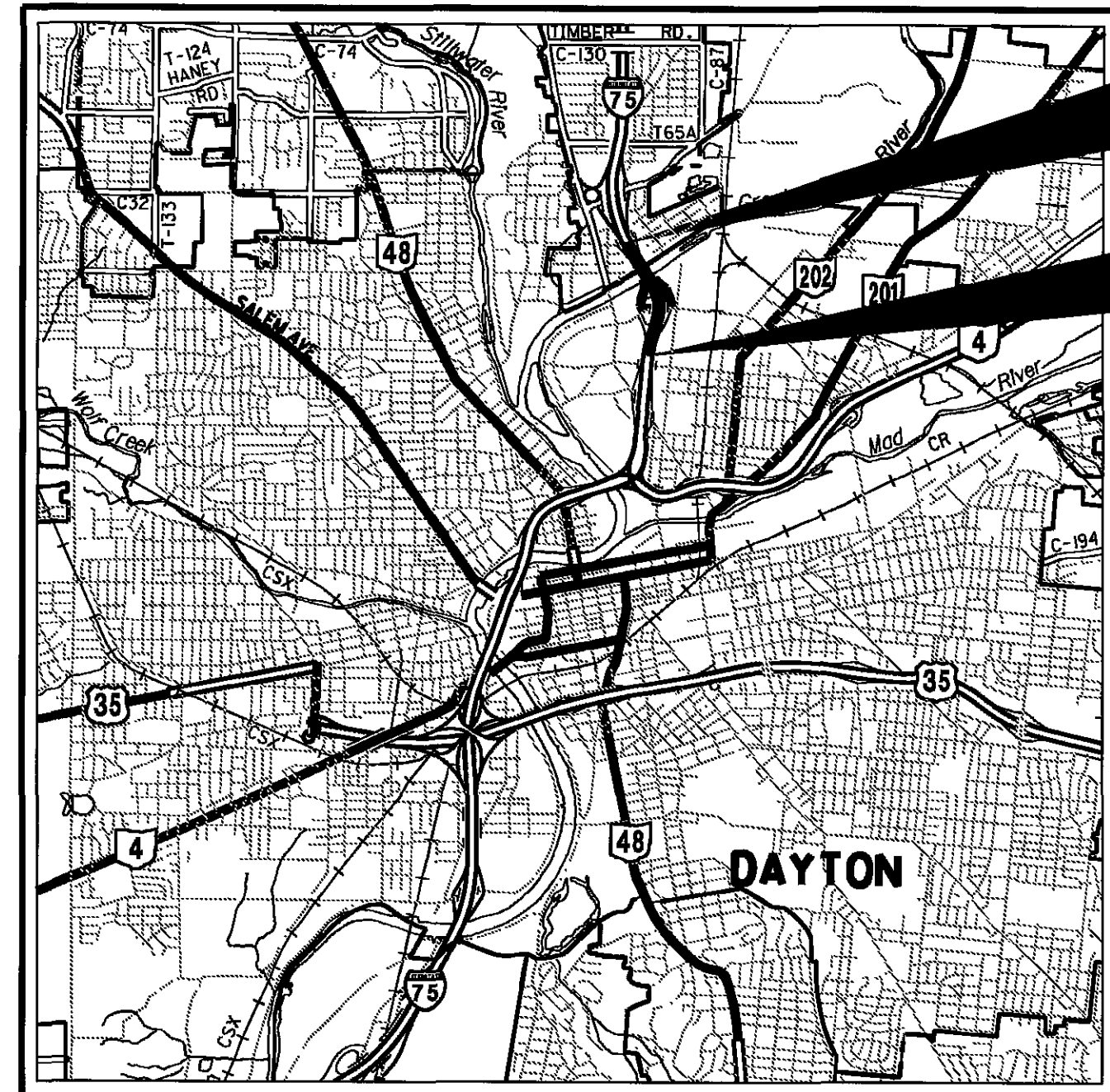
THE STANDARD SPECIFICATIONS OF THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, INCLUDING CHANGES AND SUPPLEMENTAL SPECIFICATIONS LISTED IN THE PROPOSAL SHALL GOVERN THIS IMPROVEMENT.

I HEREBY APPROVE THESE PLANS AND DECLARE THAT THE MAKING OF THIS IMPROVEMENT WILL NOT REQUIRE THE CLOSING TO TRAFFIC OF THE HIGHWAY. PROVISIONS FOR THE MAINTENANCE AND SAFETY OF TRAFFIC WILL BE AS SET FORTH ON THE PLANS AND ESTIMATES.

UNDER AUTHORITY OF SECTION 4511.21, DIVISION (I) OF THE OHIO REVISED CODE, THE REVISED PRIMA FACIE SPEED LIMITS AS INDICATED HEREIN ARE DETERMINED TO BE REASONABLE AND SAFE, AND ARE HEREBY ESTABLISHED FOR THE DURATION OF THIS PROJECT. THE PRIMA FACIE SPEED LIMIT OR LIMITS HEREBY ESTABLISHED SHALL BECOME EFFECTIVE WHEN APPROPRIATE SIGNS GIVING NOTICE THEREOF ARE ERECTED.

APPROVED Rep. Dickey, P.E., P.S. / rrw
 DATE 8-3-07 DISTRICT DEPUTY DIRECTOR

APPROVED James J. Bradley, M.E.
 DATE 9-12-07 DIRECTOR, DEPARTMENT OF TRANSPORTATION



PORTION TO BE IMPROVED: INTERSTATE & DIVIDED HIGHWAY: UNDIVIDED STATE & FEDERAL ROUTES: OTHER ROADS:

DESIGN DESIGNATION.....JR-75

CURRENT ADT (2006).....105,700
 DESIGN YEAR ADT (2026).....124,400
 DESIGN HOURLY VOLUME (2026).....11,270
 DIRECTIONAL DISTRIBUTION.....51%
 TRUCKS (24 HOUR B&C).....26%
 DESIGN SPEED.....60 MPH
 LEGAL SPEED.....55 MPH

DESIGN FUNCTIONAL CLASSIFICATION -
 URBAN INTERSTATE

DESIGN EXCEPTIONS -
 SEE SHEET 2

UNDERGROUND UTILITIES
 TWO WORKING DAYS
BEFORE YOU DIG
 CALL 1-800-362-2764 (TOLL FREE)
 OHIO UTILITIES PROTECTION SERVICE
 NON-MEMBERS
 MUST BE CALLED DIRECTLY

PLAN PREPARED BY:



5747 PERIMETER DRIVE, SUITE 240, DUBLIN, OH 43017

END PROJECT
 STA 468+30.96

BEGIN PROJECT
 STA 432+45.67

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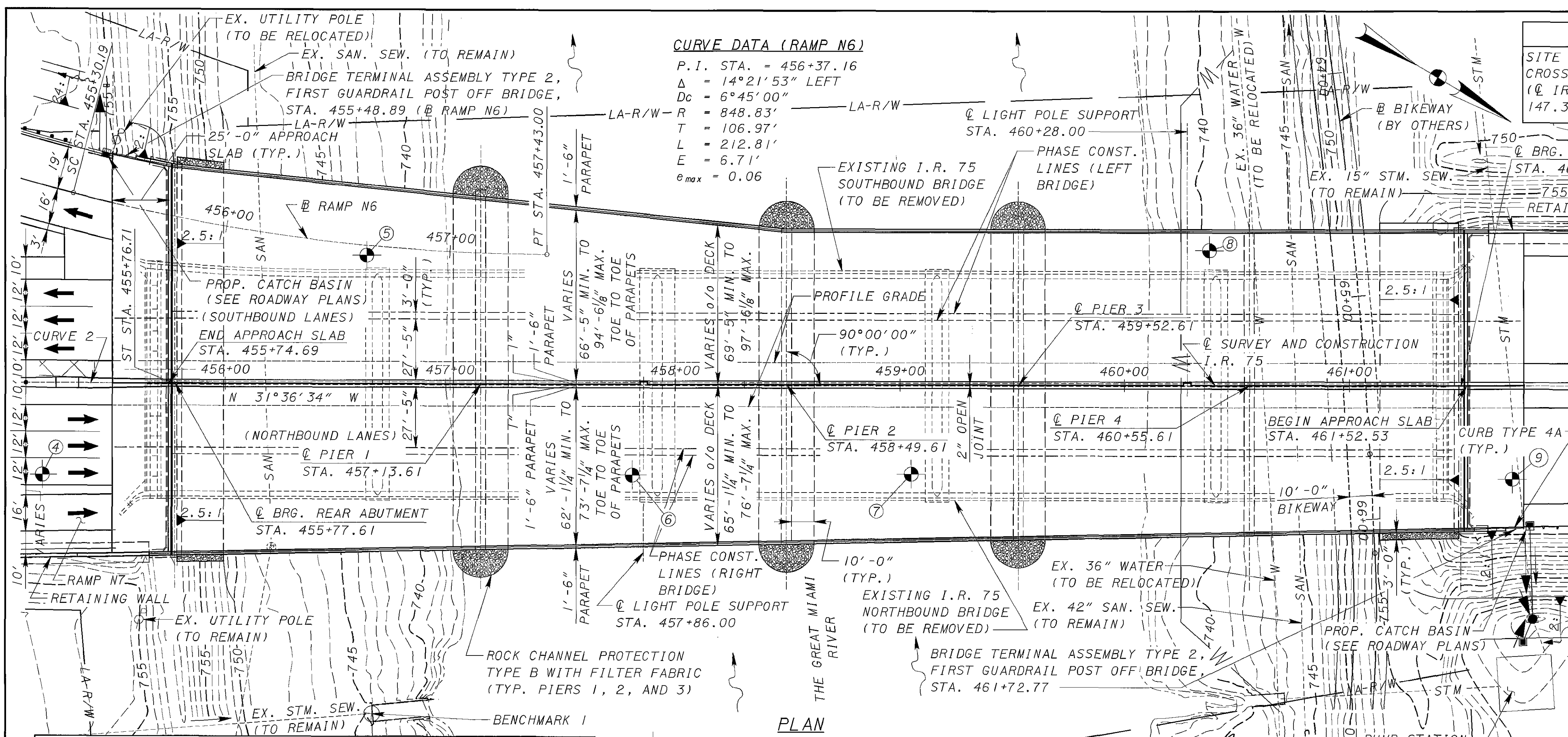
STANDARD CONSTRUCTION DRAWINGS

STANDARD CONSTRUCTION DRAWINGS											SUPPLEMENTAL SPECIFICATIONS				
BP-2.1	07/16/04	GR-5.2	1/16/04	DM-1.4	04/21/06	TC-52.10	01/19/07	MT-95.32	09/05/06	MT-105.11	10/18/02	HL-60.11	01/19/07		
BP-2.2	07/16/04	GR-5.3	1/16/04	DM-4.1	07/19/02	TC-52.20	01/19/07	MT-95.40	10/20/06	MT-110.20	10/18/02	HL-60.12	01/19/07		
BP-2.3	07/16/04	RM-3.1	04/18/03	DM-4.3	07/19/02	TC-61.10	01/19/01	MT-98.12	04/19/02	MT-120.00	03/01/00	HL-60.21	01/19/07	800	10/19/07
BP-2.4	07/16/04	RM-4.2	10/20/06	DM-4.4	07/19/02	TC-65.10	01/21/05	MT-98.13	04/19/02	HL-10.11	01/16/04	HL-60.31	01/19/07	802	04/15/05
BP-3.1	07/16/04	RM-4.3	01/19/07	TC-9.10	01/19/01	TC-65.11	01/21/05	MT-98.14	04/19/02	HL-10.12	01/19/07	A-1-69	07/19/02	832	04/25/06
BP-5.1	07/28/00	RM-4.4	01/19/07	TC-12.30	01/19/01	TC-71.10	01/19/07	MT-98.15	07/16/04	HL-10.13	01/17/03	AS-1-81	07/19/02	833	02/12/03
BP-6.1	07/28/00	RM-4.5	01/19/07	TC-21.10	01/19/01	TC-72.20	01/21/05	MT-98.16	04/19/02	HL-10.31	07/20/01	EXJ-6-06	01/20/06	872	10/30/03
F-1.1	07/16/04	CB-1.1	07/15/05	TC-21.20	01/19/01	TC-73.10	01/19/01	MT-98.17	10/18/02	HL-20.11	04/19/02	PCB-91	07/19/02	873	10/30/03
F-3.1	07/28/00	CB-2.1	07/15/05	TC-22.10	01/19/01	TC-81.10	05/01/00	MT-98.18	10/18/02	HL-20.13	01/21/05	PSID-1-99	04/20/07	898	07/21/06
F-3.3	07/28/00	CB-2.2	07/15/05	TC-22.20	01/19/01	TC-82.10	04/19/02	MT-98.19	10/18/02	HL-20.21	01/19/07	SBR-1-99	07/19/02		
GR-1.1	07/16/04	HW-1.1	01/21/05	TC-31.21	04/20/01	TC-83.10	01/19/07	MT-99.20M	17/30/95	HL-30.11	01/21/05				
GR-2.1	01/16/04	HW-2.1	04/21/06	TC-41.20	01/19/01	TC-83.20	01/19/07	MT-101.60	09/20/06	HL-30.21	01/19/07				
GR-3.1	01/19/07	HW-2.2	04/21/06	TC-41.40	07/16/04	TC-84.20	01/19/07	MT-101.70	10/18/02	HL-30.22	01/21/05				
GR-3.2	01/19/07	MH-1.2	01/20/06	TC-42.20	07/16/04	TC-85.20	05/01/00	MT-102.10	10/20/06	HL-40.10	01/19/07				
GR-4.2	01/19/07	DM-1.1	04/21/06	TC-51.11	04/20/01	MT-95.30	09/05/06	MT-102.20	09/05/06	HL-50.11	01/19/07				
GR-5.1	04/18/03	DM-1.2	10/21/05	TC-51.12	04/20/01	MT-95.31	09/05/06	MT-105.10	10/18/02	HL-50.21	01/19/07				

SPECIAL PROVISIONS

OEPA RENO DEMO
 USACE AND EPA PERMIT

FEDERAL PROJECT NO. E033(774)
 PID NO. 23828
 CONSTRUCTION PROJECT NO.
 RAILROAD INVOLVEMENT NONE
 PRESTRESSED CONCRETE OPTION
 MOT-75-14.60
 1/314



CURVE DATA (RAMP N6)

P.I. STA. = 456+37.16
 $\Delta = 14^{\circ}21'53''$ LEFT
 $D_c = 6^{\circ}45'00''$
 $R = 848.83'$
 $T = 106.97'$
 $L = 212.81'$
 $E = 6.71'$
 $e_{max} = 0.06$

BENCHMARK 1	BENCHMARK 2
SITE BM # 2 CROSS NOTCH ON TOP OF HEADWALL (@ I.R. 75) STA. 456+65.11 147.31' RT, EL. 745.33	SITE BM # 3 MCD CONTROL, MONUMENT NO. 2 (@ I.R. 75) STA. 460+86.44 287.01' RT, EL. 745.36

TRAFFIC DATA

I.R. 75
 CURRENT YEAR (2006) = 105,700 ADT
 DESIGN YEAR (2026) = 124,400 ADT
 CURRENT YEAR (2006) = 27,482 ADTT
 DESIGN YEAR (2026) = 32,344 ADTT

EXISTING STRUCTURE

TYPE: DUAL STRUCTURES - 5 SPAN CONTINUOUS STEEL RIVETED PLATE GIRDER WITH REINFORCED CONCRETE DECK SUPPORTED BY STUB ABUTMENTS ON PILES AND WALL TYPE PIERS

SPANS: 99'-6", 124'-4", 124'-4", 124'-4", 99'-6" c/c BEARINGS

ROADWAY: (2) - 40'-0" TOE TO TOE OF CURB
 LOADING: CF2000
 SKEW: NONE
 ALIGNMENT: TANGENT
 WEARING SURFACE: ASPHALT OVERLAY ON 1" MONOLITHIC CONCRETE

APPROACH SLABS: AS-1-72 (20'-0" LONG)
 DATE BUILT: 1959
 STRUCTURE FILE No.: 5708702

PROPOSED STRUCTURE

TYPE: 5 SPAN CONTINUOUS, PRESTRESSED CONCRETE I-BEAMS WITH COMPOSITE REINFORCED CONCRETE DECK SUPPORTED BY REINFORCED CONCRETE SUBSTRUCTURES ON PILES.

SPANS: 136'-0", 136'-0", 103'-0", 103'-0", 94'-0" c/c BEARINGS

ROADWAY: NORTHBOUND - VARIES, 62'-11/4" MIN. TO 73'-7/4" MAX. F/F OF PARAPET
 SOUTHBOUND - VARIES, 66'-5" MIN. TO 94'-6 1/2" MAX. F/F OF PARAPET

LOADING: HS-25 AND ALTERNATE MILITARY LOADING, FWS = 60 LBS/FT²
 SKEW: NONE
 CROWN: VARIES (NORTHBOUND), VARIES (SOUTHBOUND)
 ALIGNMENT: TANGENT
 WEARING SURFACE: 1" MONOLITHIC CONCRETE
 APPROACH SLABS: AS-1-81 (25'-0" LONG)
 LATITUDE: 39°47'29" N
 LONGITUDE: 8°11'8" W

- NOTES:**
- ALL SHEETS WITH PLAN DIMENSIONS ARE SHOWN HORIZONTAL.
 - EARTHWORK LIMITS SHOWN ARE APPROXIMATE. ACTUAL SLOPES SHALL CONFORM TO PLAN CROSS SECTIONS.
 - THE PROPOSED PROFILE GRADE IS WITHIN BRIDGE LIMITS. SEE ROADWAY PLANS FOR PAVEMENT ELEVATIONS BEYOND BRIDGE LIMITS.
 - ABBREVIATIONS:
 TYP. - TYPICAL STA. - STATION EXIST. - EXISTING
 BRG. - BEARING FIX. - FIXED FWD. - FORWARD
 ABUT. - ABUTMENT PT. - POINT EXP. - EXPANSION
 EL. - ELEVATION APPR. - APPROACH
 MCD - MIAMI CONSERVANCY DISTRICT
 N.B. - NORTHBOUND
 S.B. - SOUTHBOUND
 - FOR PHASE CONSTRUCTION LIMITS, CURVE 2 CURVE DATA AND OTHER INFORMATION NOT SHOWN, SEE GENERAL PLAN AND ELEVATION SHEET [2/55].

FOUNDATION DATA:

ALL PILES SHALL BE HP 14x73 STEEL FRICTION PILES.
 ESTIMATED PAY LENGTH:
 REAR ABUTMENT = 45 FEET
 PIER 1 = 25 FEET PIER 2 = 25 FEET
 PIER 3 = 20 FEET PIER 4 = 20 FEET
 FORWARD ABUTMENT = 45 FEET

BORING LOCATIONS

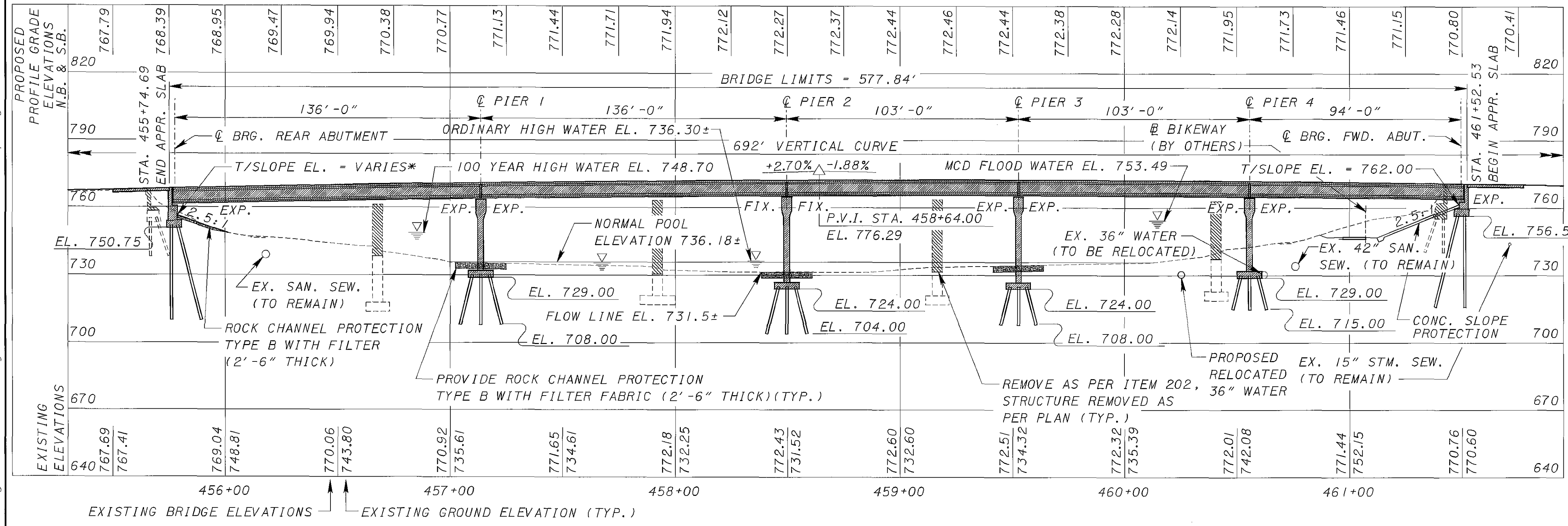
BORING	STATION	OFFSET	BORING	STATION	OFFSET
#4	455+18.71	41.22' RT.	#8	460+37.90	60.28' LT.
#5	456+62.95	57.34' LT.	#9	461+72.30	42.06' RT.
#6	457+80.95	40.79' RT.			
#7	459+04.90	40.08' RT.			

EXISTING HYDRAULIC DATA

$V_{50} = 6.65$ f/s EL. 50 = 747.61
 $V_{100} = 7.05$ f/s EL. 100 = 748.67

HYDRAULIC DATA

$Q_{50} = 35000$ cfs $V_{50} = 6.64$ f/s EL. 50 = 747.63
 DRAINAGE AREA = 0.100 = 40000 cfs $V_{100} = 7.04$ f/s EL. 100 = 748.70
 1175 sq miles MIN. CLEARANCE = 10.00' MIN. BOTTOM OF BEAM =
 FOR 100 YEAR STORM EL. 758.70 AT STA 455+95.87



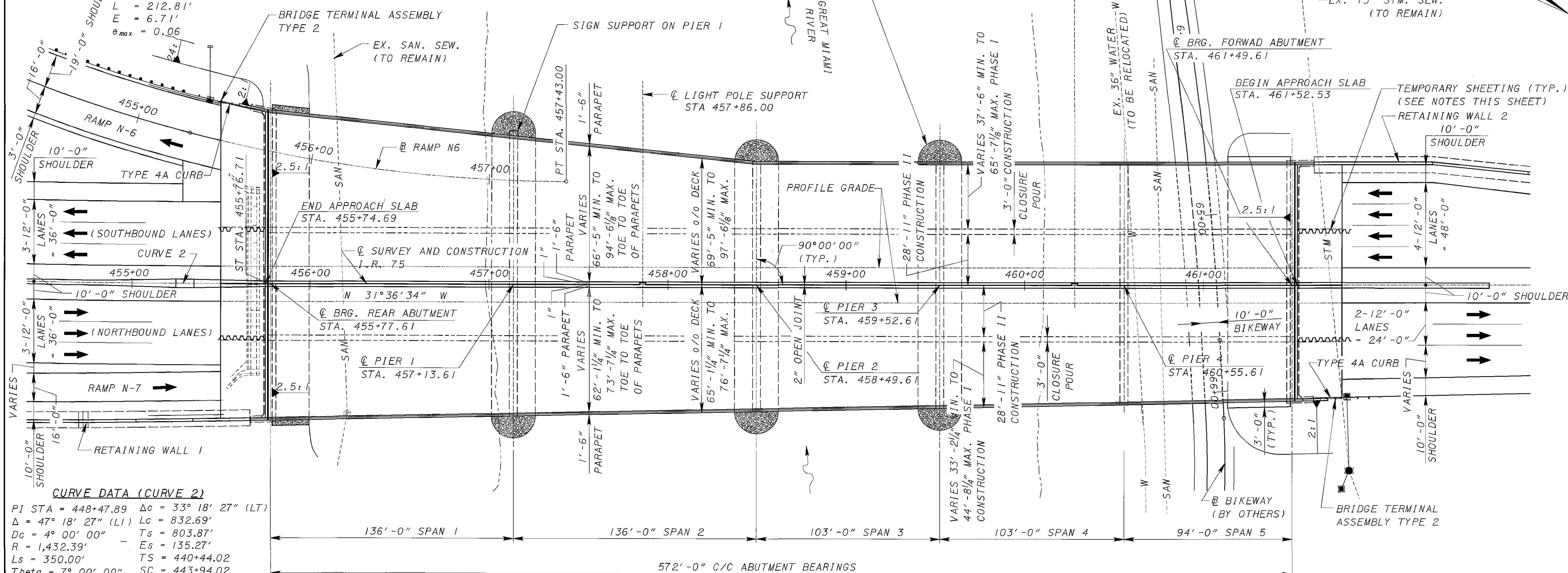
PROFILE ALONG PROFILE GRADE OF NORTHBOUND AND SOUTHBOUND I.R. 75

* SEE REAR ABUTMENT PLAN AND ELEVATION FOR ELEVATIONS

Plotted By: ecmack Date: 7/27/2007 Time: 3:41:42 PM
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CURVE DATA (RAMP N6)

P.I. STA. = 456+37.16
 $\Delta = 14^{\circ}21'53''$ LEFT
 $D_c = 6^{\circ}45'00''$
 $R = 848.83'$
 $T = 106.97'$
 $L = 212.81'$
 $E = 6.71'$
 $e_{max} = 0.06$



CURVE DATA (CURVE 2)

PI STA = 448+47.89 $\Delta_c = 33^{\circ}18'27''$ (LT)
 $\Delta = 47^{\circ}18'27''$ (LI) $L_c = 832.69'$
 $D_c = 4^{\circ}00'00''$ $T_s = 803.87'$
 $R = 1,432.39'$ $E_s = 135.27'$
 $L_s = 350.00'$ $T_S = 440+44.02$
 $\theta = 7^{\circ}00'00''$ $SC = 443+94.02$
 $LT = 233.52'$ $CS = 452+26.71$
 $ST = 116.83'$ $ST = 455+76.71$
 $x = 349.48'$ $e_{max} = EXIST$
 $y = 14.24'$
 $k = 174.91'$
 $p = 3.56'$

GENERAL PLAN

TEMPORARY SHEETING NOTES

1. THE STEEL SHEET PILING SHALL CONFORM TO ASTM A328 AND SHALL HAVE THE FOLLOWING PROPERTIES:

TEMPORARY SHEETING DATA				
LOCATION	SECTION MODULUS REQUIRED (in^3/Ft)	MINIMUM YIELD STRESS, F_y (KSI)	TOP OF SHEETING ELEVATION	BOTTOM OF SHEETING ELEVATION
REAR ABUTMENT	47.0	38.5	769.0±	725.0±
FORWARD ABUTMENT*	32.2	38.5	771.0±	735.0±

* - THE CONTRACTOR IS TO FIELD LOCATE THE EXISTING 15" DIAMETER STORM SEWER UNDER THE PROPOSED APPROACH SLAB. THE SECTION OF TEMPORARY SHEETING ABOVE THE 15" STORM SEWER WILL NEED TO BE DRIVEN TO AN ELEVATION ABOVE THE BOTTOM OF WALL ELEVATION SHOWN IN THE TABLE IN ORDER NOT TO DAMAGE THE EXISTING STORM SEWER.

2. THE TEMPORARY SHEET PILING, INCLUDING INSTALLATION, SHALL BE PAID FOR UNDER ITEM 503 - COFFERDAMS, CRIBS AND SHEETING, AS PER PLAN.

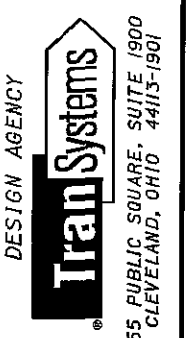
3. ALTERNATE SHEET PILING MEETING THE FOLLOWING PROPERTIES ARE PERMITTED:

ALTERNATE SHEETING		
SECTION MODULUS (in^3/Ft)	MINIMUM YIELD STRESS, F_y (KSI)	
MINIMUM YIELD STRESS, F_y (KSI)	50	60
SECTION MODULUS REQUIRED (in^3/Ft) AT REAR ABUTMENT	36.2	30.2
SECTION MODULUS REQUIRED (in^3/Ft) AT FORWARD ABUTMENT	24.8	20.7

4. THE CONTRACTOR, AT HIS OWN EXPENSE, MAY PROVIDE AN ALTERNATE DESIGN FOR THE TEMPORARY SHEETING, PER CMS 501.05 (B).

5. THE CONTRACTOR SHALL PRESERVE AND PROTECT THE POROUS BACKFILL BEHIND THE ABUTMENT AND THE AGGREGATE BASE UNDER THE APPROACH SLAB AGAINST DISPLACEMENT DURING PHASED CONSTRUCTION OPERATIONS.

6. TEMPORARY SHEETING TO BE REMOVED IN THE VICINITY OF THE PROPOSED FORWARD ABUTMENT FOOTING TO FACILITATE PHASE II CONSTRUCTION OPERATIONS.



DESIGN AGENCY
Tran Systems
 9000 W. 15th Street, Suite 100
 Overland Park, KS 66204

DATE: 5/26/05
 REVIEWED: RER
 DRAWN: CAG
 CHECKED: JDH

STRUCTURE FILE NUMBER: 5708710

GENERAL PLAN
 BRIDGE NO. MOT-75-1523
 I.R. 75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

2 / 55

237
 314

STRUCTURE GENERAL NOTES

STANDARD DRAWINGS AND SUPPLEMENTAL SPECIFICATIONS

REFER TO THE FOLLOWING STANDARD BRIDGE DRAWINGS:

A-1-69	REVISED	07-19-02
AS-1-81	REVISED	07-19-02
EXJ-6-06	REVISED	01-20-06
PCB-91	REVISED	07-19-02
PSID-1-99	REVISED	04-20-07
SBR-1-99	REVISED	07-19-02

AND TO THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS:

832	DATED	04-25-06
898	DATED	07-21-06

DESIGN SPECIFICATIONS:

THIS STRUCTURE CONFORMS TO "STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES" ADOPTED BY THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS, 17th ADDITION, AND THE ODOT BRIDGE DESIGN MANUAL.

DESIGN LOADING:

HS25 AND THE ALTERNATE MILITARY LOADING
FUTURE WEARING SURFACE (FWS) OF 60 LBS/SQ FT

DESIGN DATA:

CONCRETE CLASS QSC2 - COMPRESSIVE STRENGTH 4500 PSI (SUPERSTRUCTURE)
CONCRETE CLASS QSC1 - COMPRESSIVE STRENGTH 4000 PSI (SUBSTRUCTURE)

REINFORCING STEEL - ASTM A615 OR A996 GRADE 60
MINIMUM YIELD STRENGTH 60,000 PSI

CONCRETE FOR PRESTRESSED BEAMS - COMPRESSIVE STRENGTH (FINAL) 7000 PSI
COMPRESSIVE STRENGTH (RELEASE) 5000 PSI

DIAPHRAGM CONCRETE - COMPRESSIVE STRENGTH 4500 PSI

PRESTRESSING STRAND - AREA = 0.167 SQ. IN.
ULTIMATE STRENGTH = 270 KSI
INITIAL STRESS = 202.5 KSI (LOW RELAXATION STRANDS)

STRUCTURAL STEEL - ASTM A709 GRADE 50 - YIELD STRENGTH 50,000 PSI

HP PILES - ASTM A709 GRADE 50 - YIELD STRENGTH 50,000 PSI

DECK PROTECTION METHOD:

EPOXY COATED REINFORCING STEEL
2 1/2" CONCRETE COVER

MONOLITHIC WEARING SURFACE:

MONOLITHIC WEARING SURFACE IS ASSUMED, FOR DESIGN PURPOSES, TO BE 1" THICK.

ITEM 202, PORTIONS OF STRUCTURE REMOVED, AS PER PLAN:

REMOVE ABUTMENTS AND PIERS TO THE ELEVATIONS SHOWN ON THE REMOVAL DETAILS SHEET.

ITEM 203, EMBANKMENT, AS PER PLAN:

PLACE AND COMPACT EMBANKMENT MATERIAL IN 6 INCH LIFTS FOR THE CONSTRUCTION OF THE APPROACH EMBANKMENT BETWEEN STATIONS 454+74.00 TO 462+53.00.

FRICTION TYPE PILES:

THE PILES ESTIMATED LENGTHS, ORDER LENGTHS, ULTIMATE BEARING VALUES, AND NUMBER OF DYNAMIC LOAD TESTING ITEMS ARE AS LISTED BELOW. PILES AT PIERS 1 THRU 3 INCLUDE AN ADDITIONAL 8 TONS PER PILE OF ULTIMATE BEARING DUE TO THE POSSIBILITY OF LOSING FRICTIONAL RESISTANCE DUE TO SCOUR.

REAR ABUTMENT PILES:

48 HP14x73 PILES 50 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 184 TONS PER PILE
1 DYNAMIC LOAD TESTING ITEMS

PIER 1 PILES:

78 HP14x73 PILES 30 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 198 TONS PER PILE
1 DYNAMIC LOAD TESTING ITEMS
1 STATIC LOAD TESTING ITEMS, 3 RESTRIKES

PIER 2 PILES:

66 HP14x73 PILES 30 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 192 TONS PER PILE

PIER 3 PILES:

66 HP14x73 PILES 25 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 198 TONS PER PILE

PIER 4 PILES:

66 HP14x73 PILES 25 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 184 TONS PER PILE
1 DYNAMIC LOAD TESTING ITEMS

FORWARD ABUTMENT PILES:

39 HP14x73 PILES 50 FEET LONG ORDER LENGTH
ULTIMATE BEARING VALUE = 144 TONS PER PILE
1 DYNAMIC LOAD TESTING ITEMS

PILE SPLICES:

IN LIEU OF USING FULL PENETRATION BUTT WELDS SPECIFIED IN CMS 507.09 TO SPLICE STEEL H-PILES, THE CONTRACTOR MAY USE A MANUFACTURED H-PILE SPLICER. FURNISH SPLICERS FROM THE FOLLOWING MANUFACTURER: ASSOCIATED PILE AND FITTING CORPORATION, 262 RUTHERFORD BLVD., CLIFTON, NEW JERSEY 07014. INSTALL AND WELD THE SPLICER TO THE PILE SECTIONS IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN ASSEMBLY PROCEDURE SUPPLIED TO THE ENGINEER BEFORE THE WELDING IS PERFORMED.

BATTERED PILES:

THE BLOW COUNT FOR BATTERED PILES SHALL BE THE BLOW COUNT DETERMINED FOR VERTICAL PILES OF THE SAME ULTIMATE BEARING VALUE DIVIDED BY AN EFFICIENCY FACTOR (D). COMPUTE THE EFFICIENCY FACTOR (D) AS FOLLOWS:

$$D = \frac{1-UG}{\sqrt{(1+G^2)}}$$

U = COEFFICIENT OF FRICTION, WHICH IS ESTIMATED AT 0.05 FOR DOUBLE-ACTING AIR OPERATED OR DIESEL HAMMERS; 0.1 FOR SINGLE-ACTING AIR OPERATED OR DIESEL HAMMERS; AND 0.2 FOR DROP HAMMERS.

G = RATE OF BATTER (1/3, 1/4, ETC.)

STATIC LOAD TEST:

PERFORM DYNAMIC TESTING ON THE FIRST TWO PRODUCTION PILES TO DETERMINE THE REQUIRED BLOW COUNT FOR THE SPECIFIED ULTIMATE BEARING VALUE. PERFORM THE STATIC LOAD TEST ON EITHER PILE. DO NOT OVER-DRIVE THE SELECTED PILE. DRIVE THE THIRD AND FOURTH PRODUCTION PILES TO 75% AND 85% OF THE DETERMINED BLOW COUNT, RESPECTIVELY. THE TEST PILES AND THE REDUCED CAPACITY PILES SHALL NOT BE BATTERED. AFTER INSTALLATION OF THE FIRST FOUR PRODUCTION PILES, CEASE ALL DRIVING OPERATIONS ON PILING REPRESENTED BY THE STATIC LOAD TESTING FOR A MINIMUM OF 7 DAYS. AFTER THE WAITING PERIOD, PERFORM PILE RESTRIKES ON THE STATIC LOAD TEST PILE AND EACH REDUCED CAPACITY PILE. THE ENGINEER WILL REVIEW THE RESULTS OF THE PILE RESTRIKES AND ESTABLISH THE DRIVING CRITERIA FOR THE REMAINING PILING REPRESENTED BY THE TESTING.

FOR SUBSEQUENT STATIC LOAD TESTS, UPON COMPLETION OF A 10,000 FOOT INCREMENT OF DRIVEN LENGTH, REPEAT THE ABOVE PROCEDURE FOR THE INITIAL STATIC LOAD TEST. IF NECESSARY, THE ENGINEER WILL REVISE THE DRIVING CRITERIA FOR THE REMAINING PILING ACCORDINGLY.

WHEN PERFORMING THE RESTRIKE, IF THE PILE HAS NOT REACHED THE BLOW COUNT DETERMINED FOR THE PLAN SPECIFIED ULTIMATE BEARING VALUE, CONTINUE DRIVING THE PILE UNTIL THIS CAPACITY IS ACHIEVED.

MAINTENANCE OF TRAFFIC:

I.R. 75 TRAFFIC WILL BE MAINTAINED AT ALL TIMES. FOR MAINTENANCE OF TRAFFIC NOTES, PERMITTED LANE CLOSURE AND DETAILS, REFER TO ROADWAY PLANS AND STRUCTURE PHASE CONSTRUCTION DETAILS. NIGHTTIME TEMPORARY CLOSURES MAY BE REQUIRED FOR GIRDER ERECTION.

UTILITY LINES:

THE UTILITY(IES) SHALL BORE ALL EXPENSE INVOLVED IN RELOCATING (INSTALLING) THE AFFECTED UTILITY LINES. THE CONTRACTOR AND UTILITY(IES) ARE TO COOPERATE BY ARRANGING THEIR WORK IN SUCH A MANNER THAT INCONVENIENCE TO EITHER WILL BE HELD TO MINIMUM.

EXISTING STRUCTURE VERIFICATION:

DETAILS AND DIMENSIONS SHOWN ON THESE PLANS PERTAINING TO THE EXISTING STRUCTURE HAVE BEEN OBTAINED FROM PLANS OF THE EXISTING STRUCTURE AND FROM THE FIELD OBSERVATIONS AND MEASUREMENTS. CONSEQUENTLY, THEY ARE INDICATIVE OF THE EXISTING STRUCTURE AND THE PROPOSED WORK BUT THEY SHALL BE CONSIDERED TENTATIVE AND APPROXIMATE. THE CONTRACTOR IS REFERRED TO CMS SECTIONS 102.05 AND 105.02.

BASE CONTRACT BID PRICES UPON A RECOGNITION OF THE UNCERTAINTIES DESCRIBED ABOVE AND UPON A PREBID EXAMINATION OF THE EXISTING STRUCTURE. HOWEVER, THE DEPARTMENT WILL PAY FOR ALL PROJECT WORK BASED UPON ACTUAL DETAILS AND DIMENSIONS THAT HAVE BEEN VERIFIED IN THE FIELD.

ITEM 509, EPOXY-COATED REINFORCING STEEL, AS PER PLAN:

IN ADDITION TO THE PROVISIONS OF ITEM 509, FIELD BEND AND/OR FIELD CUT THE REINFORCING STEEL DESIGNATED IN THE PLANS, AS NECESSARY, IN ORDER TO MAINTAIN THE REQUIRED CLEARANCES AND BAR SPACINGS. REPAIR ALL DAMAGE TO THE EPOXY COATING, AS A RESULT OF THIS WORK, ACCORDING TO 709.00.

MECHANICAL CONNECTORS

AN APPROVED TYPE OF MECHANICAL CONNECTOR FOR REINFORCING BARS SHALL BE PROVIDED. INSTALLATION OF CONNECTORS SHALL CONFORM WITH MANUFACTURER'S PROCEDURES. MECHANICAL CONNECTORS SHALL BE CAPABLE OF DEVELOPING 125% OF THE YIELD STRENGTH OF THE BARS CONNECTED. ALL MECHANICAL CONNECTORS SHALL BE EPOXY COATED. COATING FOR BOTH CONNECTORS AND BARS SHALL CONFORM TO THE SAME SPECIFICATIONS. COATINGS WHICH HAVE BEEN DAMAGED OR WHICH OTHERWISE DO NOT MEET SPECIFICATIONS WITH RESPECT TO COLOR, CONTINUITY, AND UNIFORMITY MAY BE REPAIRED AS DIRECTED BY THE ENGINEER OR THEY SHALL BE REPLACED WITH MATERIAL WHICH MEETS THE SPECIFICATIONS. CONNECTORS SHALL CONFORM WITH ITEM 509 AND SHALL BE CONSIDERED INCIDENTAL TO THE BID PRICE FOR ITEM 509-EPOXY COATED REINFORCING STEEL.

ITEM 512, SEALING OF CONCRETE SURFACES (EPOXY-URETHANE):

THE CONCRETE SEALING MATERIAL SHALL BE THE "LIGHT NEUTRAL" COLOR MEETING THE FEDERAL COLOR STANDARD NO. 17778 AS PER THE DETAILS ON THE PLANS.

ITEM 898, QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (APPROACH SLAB), 15", AS PER PLAN:

FURNISH APPROACH SLABS CONFORMING TO CMS 526 EXCEPT CONCRETE SHALL BE IN ACCORDANCE WITH SUPPLEMENTAL SPECIFICATION 898, QC/QA CONCRETE, CLASS QSC2. THE ACCEPTED QUANTITIES SHALL INCLUDE: CONCRETE, CURBS, PARAPET AND MEDIAN BARRIER CONCRETE, REINFORCING STEEL, JOINT FILLERS, JOINT SEALERS, JOINT SEALS, AND WATERPROOFING ON THE APPROACH SLABS AND ON THE BACKWALLS OF THE ABUTMENTS. THE DEPARTMENT WILL MEASURE APPROACH SLABS BY THE NUMBER OF SQUARE YARDS. THE DEPARTMENT WILL INITIALLY PAY THE FULL BID PRICE TO THE CONTRACTOR UPON COMPLETING THE WORK. THE DEPARTMENT WILL CALCULATE THE FINAL ADJUSTED PAYMENT ACCORDING TO 898.17 AND INCLUDE APPROACH SLAB CONCRETE AND DECK CONCRETE IN THE SAME LOT TO DETERMINE FINAL PAY FACTORS.

ITEM 898, QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (PARAPET), AS PER PLAN:

THE DEPARTMENT WILL CALCULATE THE FINAL ADJUSTED PAYMENT ACCORDING TO 898.17 AND INCLUDE APPROACH SLAB CONCRETE AND PARAPET IN THE SAME LOT TO DETERMINE FINAL PAY FACTORS.

ITEM 898, QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (DECK), AS PER PLAN:

THE DEPARTMENT WILL CALCULATE THE FINAL ADJUSTED PAYMENT ACCORDING TO 898.17 AND INCLUDE APPROACH SLAB CONCRETE AND DECK CONCRETE IN THE SAME LOT TO DETERMINE FINAL PAY FACTORS.

INDEX OF SHEETS:

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GENERAL NOTES [3/55]
ESTIMATED QUANTITIES [4/55]
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REMOVAL DETAILS [7/55]
BRIDGE PHASE CONSTRUCTION PLANS AND NOTES [8/55]
TEMPORARY CAUSEWAY AND STAGE CONSTRUCTION DETAILS [9/55] THRU [12/55]
PILE LAYOUT PLAN [13/55] AND [14/55]
REAR ABUTMENT DETAILS [15/55] THRU [18/55]
FORWARD ABUTMENT DETAILS [19/55] THRU [21/55]
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FRAMING PLAN [27/55] AND [28/55]
BEAM DETAILS [29/55] AND [30/55]
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DIAPHRAGM DETAILS [32/55] THRU [37/55]
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TRANSVERSE SECTION [39/55]
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PARAPET DETAILS [47/55] AND [48/55]
BEARING DETAILS [49/55]
STRIP SEAL EXPANSION JOINT DETAILS [50/55]
APPROACH SLAB DETAILS [51/55]
REINFORCING STEEL LIST [52/55] THRU [55/55]

MADE BY NFF DATE 04/25/05
 CHECK'D BY ECM DATE 07/26/07

ESTIMATED QUANTITIES

STRUCTURE FILE NUMBER: 5708710

ITEM	EXTENSION	TOTAL*	UNIT	DESCRIPTION	REAR ABUTMENT	FORWARD ABUTMENT	PIERS	SUPERSTRUCTURE	GENERAL	AS PER PLAN REFERENCE SHEET NUMBER
202	11203	LUMP		PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN					LUMP	3/55
202	22900	442	SQ YD	APPROACH SLAB REMOVED					442	
503	11101	LUMP		COFFERDAMS, CRIBS AND SHEETING, AS PER PLAN					LUMP	3/55, 9-12/55
503	21300	LUMP		UNCLASSIFIED EXCAVATION					LUMP	
505	11100	LUMP		PILE DRIVING EQUIPMENT MOBILIZATION					LUMP	
506	11100	LUMP		STATIC LOAD TEST					LUMP	3/55
507	00300	11970	FT	STEEL PILES HP 14x73, FURNISHED	2400	1950	7620			
507	00350	10155	FT	STEEL PILES HP 14x73, DRIVEN	2160	1755	6240			
509	10001	1447570	POUND	EPOXY COATED REINFORCING STEEL, AS PER PLAN	33011	24678	377248	1012633		3/55
512	10100	8410	SQ YD	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	319	218	4458	3415		
512	10300	896	SQ YD	SEALING CONCRETE BRIDGE DECKS WITH HMWM RESIN				896		
512	33000	25	SQ YD	TYPE 2 WATERPROOFING	14	11				
515	15050	96	EACH	DRAPED STRAND PRESTRESSED CONCRETE BRIDGE I-BEAM MEMBERS, LEVEL 3, TYPE 4 MOD. (72")				96		
515	20000	258	EACH	INTERMEDIATE DIAPHRAGMS				258		
516	11210	308	FT	STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL	174	134				
516	13600	75	SQ FT	1" PREFORMED EXPANSION JOINT FILLER	45	30				
516	13900	331	SQ FT	2" PREFORMED EXPANSION JOINT FILLER	8	8			315	
516	43200	38	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (10" x 22" x 2.12")			38			
516	43300	36	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (12" x 18" x 3.62")			36			
516	43400	36	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (13.5" x 18.5" x 4.46")			36			
516	43400	42	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (14" x 22" x 4.51")			42			
516	43400	22	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (13.5" x 22" x 4.51")	22					
516	43400	18	EACH	ELASTOMERIC BEARING WITH INTERNAL LAMINATES ONLY (NEOPRENE) (15.5" x 15.5" x 4.89")		18				
518	21200	332	CU YD	POROUS BACKFILL WITH FILTER FABRIC	201	131				
518	40000	328	FT	6" PERFORATED CORRUGATED PLASTIC PIPE	184	144				
518	40010	170	FT	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	66	104				
523	20000	4	EACH	DYNAMIC LOAD TESTING	1	1	2			
523	20500	3	EACH	RESTRIKING			3			
601	20000	6	SQ YD	CRUSHED AGGREGATE SLOPE PROTECTION	3	3				
601	21000	631	SQ YD	CONCRETE SLOPE PROTECTION		631				
601	32100	371	CU YD	ROCK CHANNEL PROTECTION, TYPE B WITH FILTER	371					
601	32104	930	CU YD	ROCK CHANNEL PROTECTION, TYPE B WITH FABRIC FILTER			930			
898	10201	2844	CU YD	QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (DECK), AS PER PLAN				2844		3/55
898	10705	851	SQ YD	QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (APPROACH SLAB), T=15", AS PER PLAN					851	3/55 & 15/55
898	11001	394	CU YD	QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (PARAPET), AS PER PLAN				394		3/55
898	20100	2102	CU YD	QC/QA CONCRETE, CLASS QSC1, SUBSTRUCTURE (PIER ABOVE FOOTING)			2102			
898	20150	457	CU YD	QC/QA CONCRETE, CLASS QSC1, SUBSTRUCTURE (ABUTMENT)	290	167				
898	20300	1108	CU YD	QC/QA CONCRETE, CLASS QSC1, SUBSTRUCTURE (FOOTING)	147	117	844			

* - ALL STRUCTURAL ITEMS PAID FOR WITH FEDERAL AND STATE FUNDS



DATE 05/26/05
 REVIEWED RER
 STRUCTURE FILE NUMBER 5708710

DESIGNED NFF
 CHECKED MLR

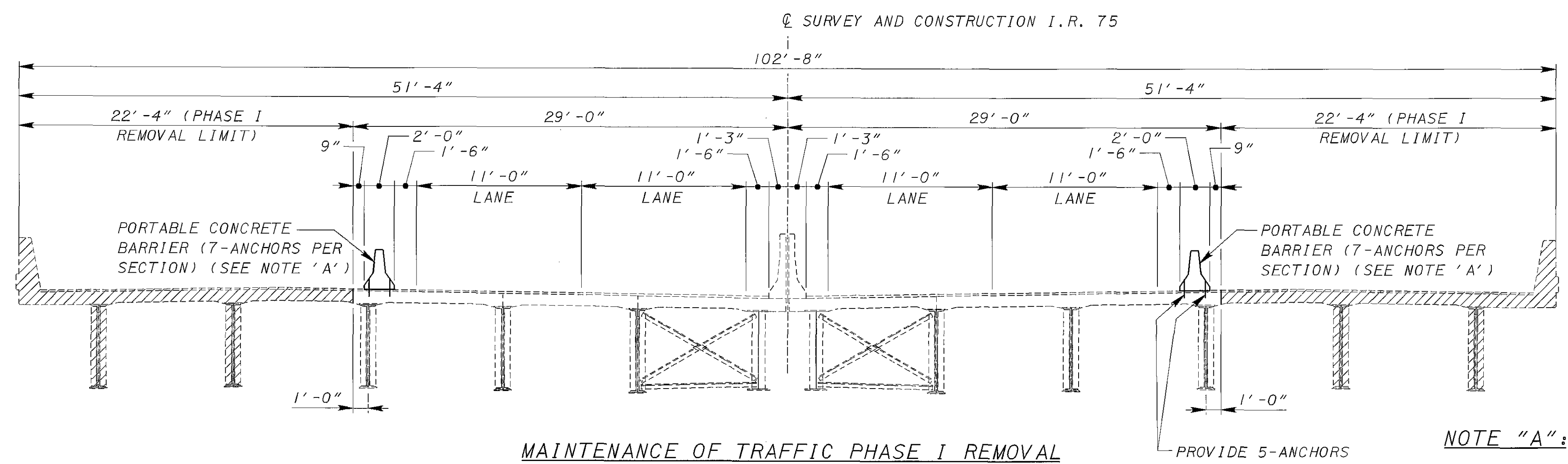
ESTIMATED QUANTITIES
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 I.R. 75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

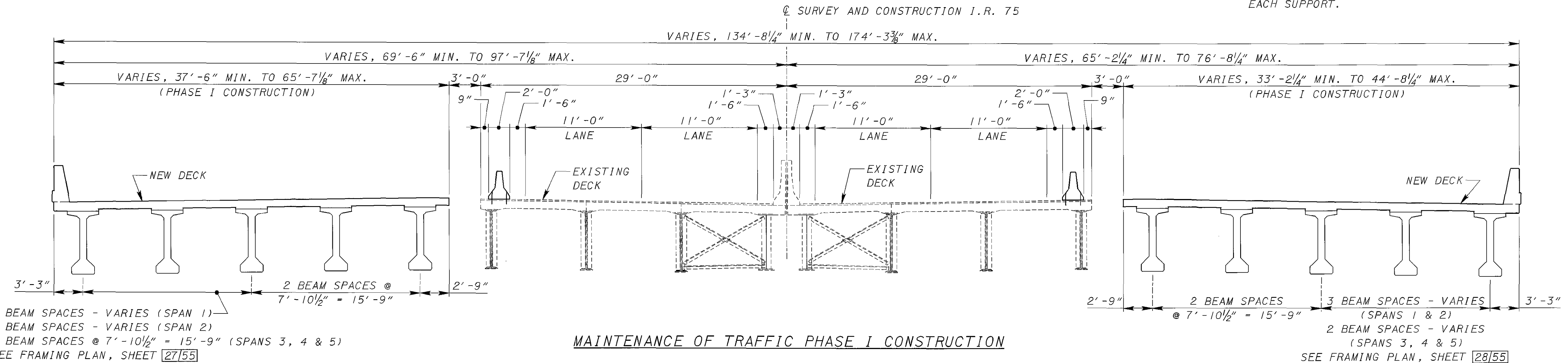
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NOTE "A":
 ALL BARRIER SEGMENTS SHALL BE FASTENED TO THE BRIDGE DECK USING ONE INCH DIAMETER HIGH STRENGTH THROUGH BOLTS OR APPROVED RESIN ANCHORS. WHEN RESIN ANCHORS ARE USED THEY MUST BE EMBEDDED A MINIMUM OF 6 1/2" INTO FIRM CONCRETE. ALL ANCHORS SHALL BE PLACED SYMMETRICAL ABOUT THE CENTER OF EACH SUPPORT.



5 BEAM SPACES - VARIES (SPAN 1)
 3 BEAM SPACES - VARIES (SPAN 2)
 2 BEAM SPACES @ 7'-10 1/2" = 15'-9" (SPANS 3, 4 & 5)
 SEE FRAMING PLAN, SHEET 27/55

2'-9" 2 BEAM SPACES @ 7'-10 1/2" = 15'-9" 3 BEAM SPACES - VARIES (SPANS 1 & 2) 2 BEAM SPACES - VARIES (SPANS 3, 4 & 5) SEE FRAMING PLAN, SHEET 28/55

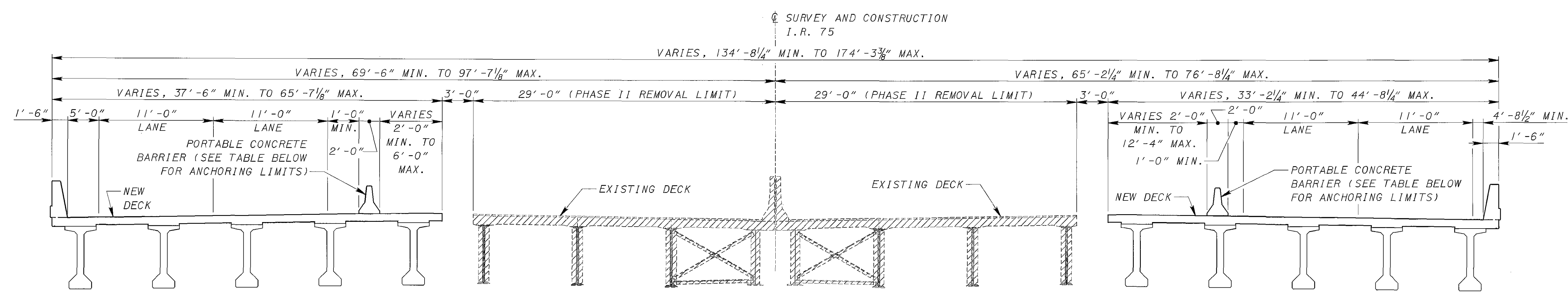
SUGGESTED CONSTRUCTION SEQUENCE:

- PHASE I**
1. DIRECT I.R. 75 TRAFFIC TO OCCUPY THE TEMPORARY LANES SHOWN IN MAINTENANCE OF TRAFFIC PHASE I.
 2. INSTALL PORTABLE CONCRETE BARRIER AS SHOWN ON THE PLANS.
 3. REMOVE EXISTING DECK TO THE LIMIT SHOWN IN MAINTENANCE OF TRAFFIC PHASE I REMOVAL.
 4. INSTALL TEMPORARY SHEETING AT ABUTMENTS.
 5. REMOVE PART OF EXISTING SUBSTRUCTURE UNITS AS NECESSARY.
 6. CONSTRUCT PART OF NEW SUBSTRUCTURE UNITS.
 7. ERECT NEW BEAMS.
 8. CONSTRUCT NEW DECK AND PARAPET AS SHOWN ON MAINTENANCE OF TRAFFIC PHASE I CONSTRUCTION.

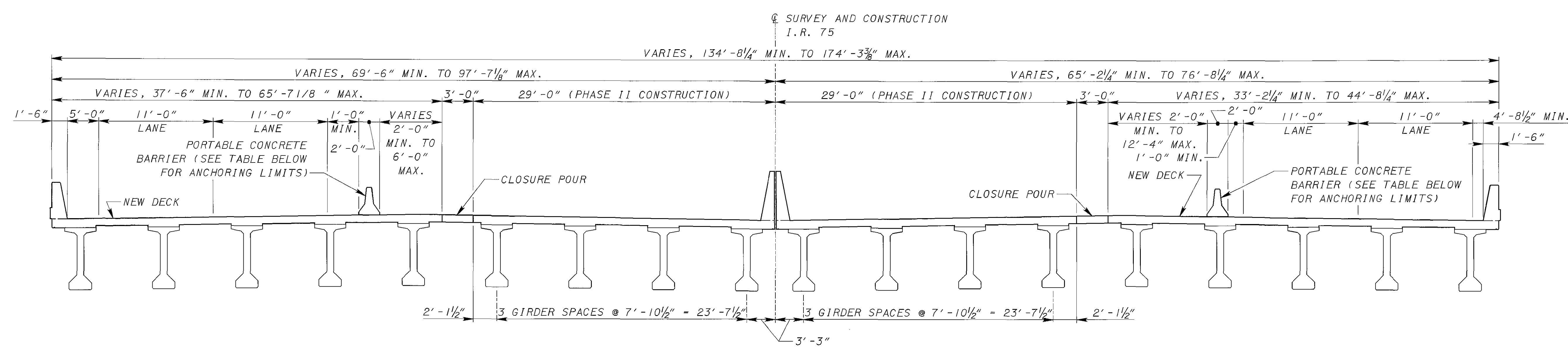
- PHASE II**
1. INSTALL PORTABLE CONCRETE BARRIER AS SHOWN ON THE PLANS.
 2. DIRECT I.R. 75 TRAFFIC TOWARD NEWLY CONSTRUCTED DECK.
 3. REMOVE REMAINING EXISTING DECK TO THE LIMIT SHOWN IN MAINTENANCE OF TRAFFIC PHASE II REMOVAL.
 4. REMOVE REMAINING PARTS OF EXISTING SUBSTRUCTURE UNITS AS NECESSARY.
 5. CONSTRUCT REMAINING PARTS OF NEW SUBSTRUCTURE UNITS.
 6. REMOVE TEMPORARY SHEETING AS NECESSARY.
 7. ERECT REMAINING NEW BEAMS.
 8. CONSTRUCT NEW DECK AND MEDIAN BARRIER AS SHOWN ON MAINTENANCE OF TRAFFIC PHASE II CONSTRUCTION.

9. PLACE CLOSURE DECK SECTION.
10. RESUME NORMAL TRAFFIC.

NOTES:
 - INDICATES REMOVAL



MAINTENANCE OF TRAFFIC PHASE II REMOVAL



MAINTENANCE OF TRAFFIC PHASE II CONSTRUCTION

PORTABLE CONCRETE BARRIER - ANCHORING REQUIREMENTS			
SOUTHBOUND		NORTHBOUND	
DISTANCE	NO. OF ANCHORS REQUIRED PER SEGMENT	DISTANCE	NO. OF ANCHORS REQUIRED PER SEGMENT
REAR ABUTMENT TO STA. 459+70.28	NONE REQUIRED	REAR ABUTMENT TO STA. 459+50.42	NONE REQUIRED
STA. 459+70.28 TO FORWARD ABUTMENT	2*	STA. 459+50.42 TO FORWARD ABUTMENT	2*

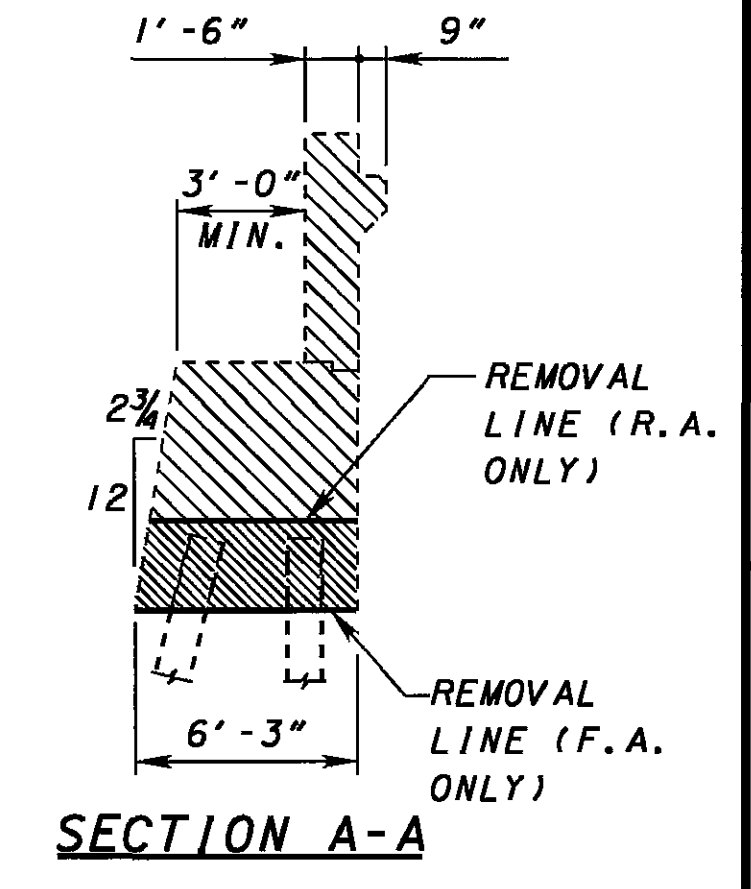
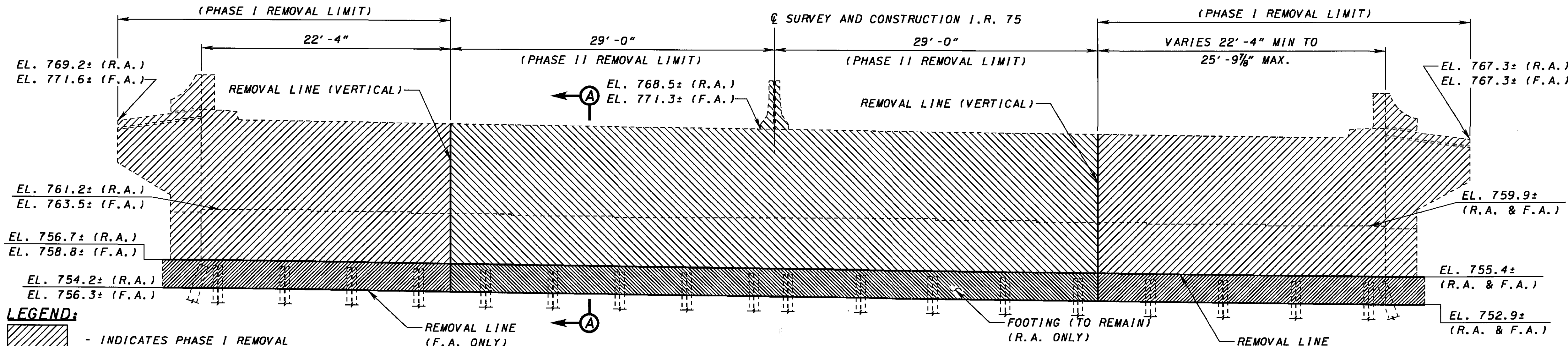
* CONTRACTOR TO AVOID DRILLING THROUGH SLAB REINFORCEMENTS

NOTES:

- INDICATES REMOVAL

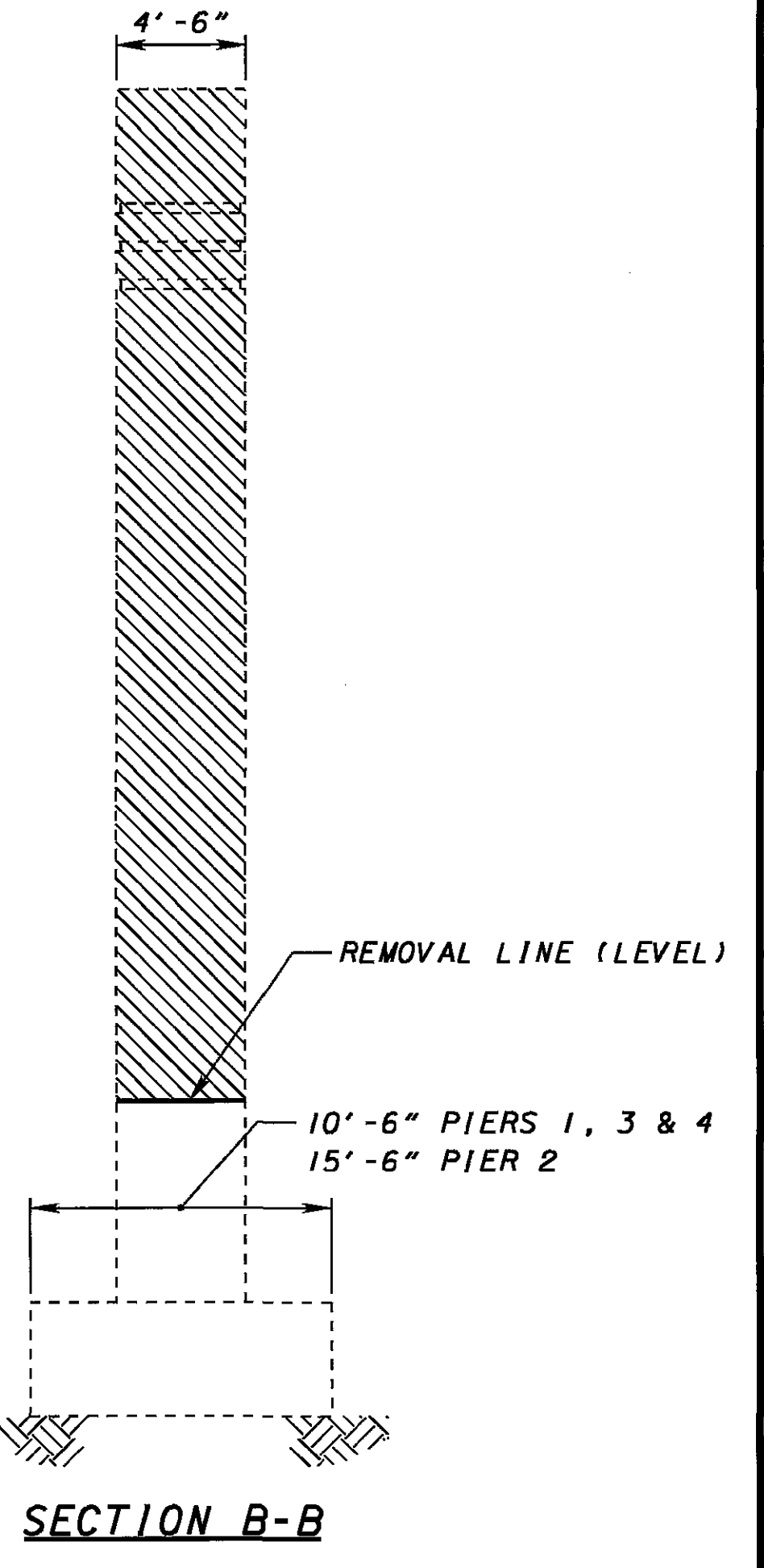
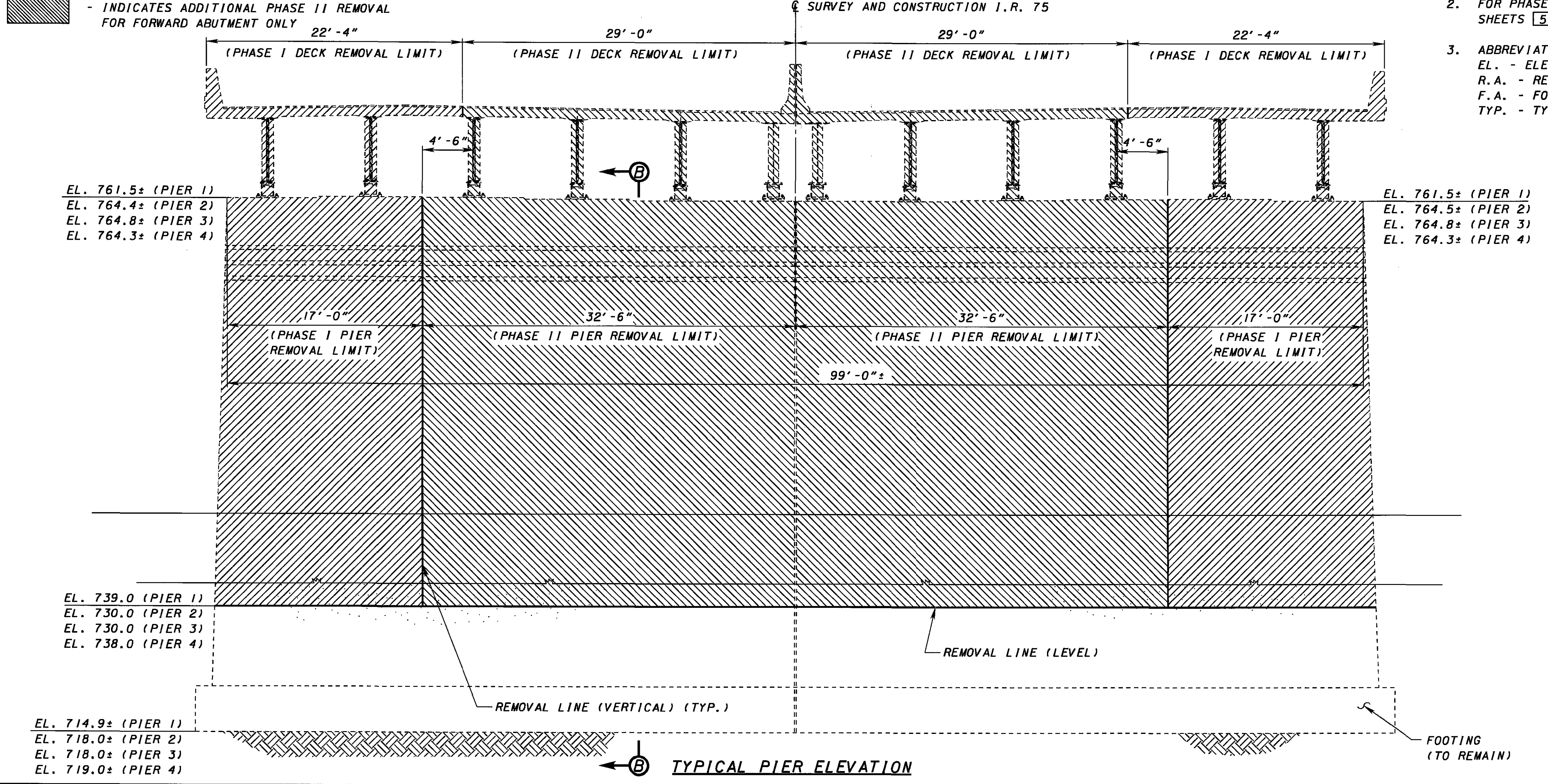
1. FOR SUGGESTED CONSTRUCTION SEQUENCE, SEE SHEET 5/55.

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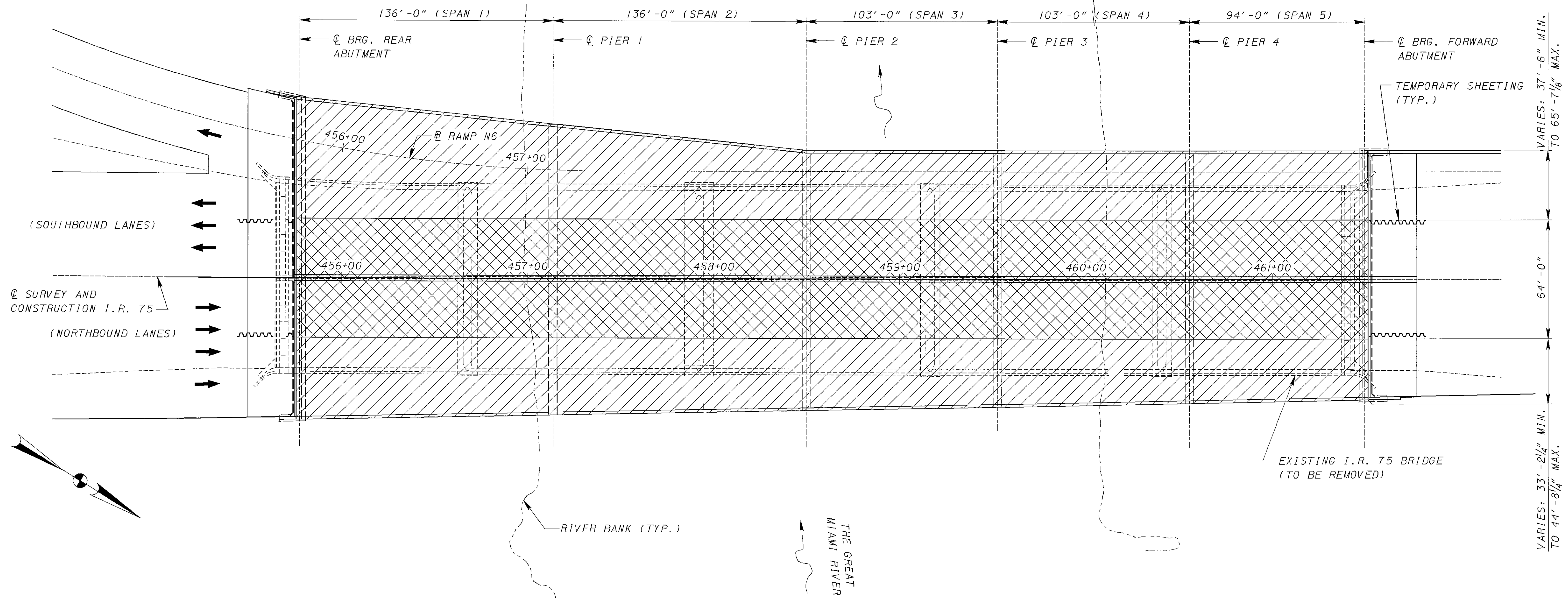
- LEGEND:**
- INDICATES PHASE I REMOVAL
 - INDICATES ADDITIONAL PHASE I REMOVAL FOR FORWARD ABUTMENT ONLY
 - INDICATES PHASE II REMOVAL
 - INDICATES ADDITIONAL PHASE II REMOVAL FOR FORWARD ABUTMENT ONLY

- NOTES:**
- AREAS TO BE REMOVED SHALL BE PAID FOR UNDER ITEM 202 - PORTIONS OF STRUCTURE REMOVED, AS PER PLAN.
 - FOR PHASE CONSTRUCTION DETAILS, SEE SHEETS [5/55] AND [6/55].
 - ABBREVIATIONS:
EL. - ELEVATION
R.A. - REAR ABUTMENT
F.A. - FORWARD ABUTMENT
TYP. - TYPICAL



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DESIGNED BY: BTA CHECKED: JDH DRAWN: BTA REVISIONS:
DATE: 05/26/05 HFF: 5708710 STRUCTURE FILE NUMBER: 5708710
REMOVAL DETAILS BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION) I.R. 75 OVER THE GREAT MIAMI RIVER
MOT-75-14.60 PID 23828
7/55
242 314



BRIDGE PHASE CONSTRUCTION PLAN

CONSTRUCTION SEQUENCE

THE PROPOSED WORK SHALL CONSIST OF THE REMOVAL OF THE EXISTING I.R. 75 BRIDGE OVER THE GREAT MIAMI RIVER AND THE CONSTRUCTION OF THE REPLACEMENT BRIDGE IN PHASES. REMOVAL AND CONSTRUCTION OPERATIONS ARE TO BE PERFORMED WHILE MAINTAINING TRAFFIC AND WATER FLOW. THE PERFORMANCE OF ALL PHASES OF WORK MUST BE COORDINATED TO SATISFY THE PROJECT MAINTENANCE OF TRAFFIC AND SAFETY REQUIREMENTS. SEE THE PROJECT MAINTENANCE OF TRAFFIC PLANS FOR ADDITIONAL MAINTENANCE OF TRAFFIC REQUIREMENTS.

PHASE I CONSTRUCTION:

- BUILD STAGE 1 OF THE TEMPORARY CONSTRUCTION ACCESS AS SHOWN ON SHEET [9/55] TO DEMOLISH AND CONSTRUCT THE EXTERIOR PORTIONS OF THE I.R. 75 BRIDGE IN SPANS 3 THROUGH 5.
- CONSTRUCT THE EXTERIOR PORTIONS OF THE SUBSTRUCTURE FOR PIERS 2 THROUGH 4 AND THE FORWARD ABUTMENT.
- SET THE SUPERSTRUCTURE BEAMS FOR THE EXTERIOR PORTIONS OF THE BRIDGE FOR SPANS 3 THROUGH 5.
- REMOVE STAGE 1 AND BUILD STAGE 2 OF THE TEMPORARY CONSTRUCTION ACCESS AS SHOWN ON SHEET [10/55] TO DEMOLISH AND CONSTRUCT THE EXTERIOR PORTIONS OF THE I.R. 75 BRIDGE IN SPANS 1 AND 2.
- CONSTRUCT THE EXTERIOR PORTIONS OF THE SUBSTRUCTURE FOR PIER 1 AND THE REAR ABUTMENT.
- SET THE SUPERSTRUCTURE BEAMS FOR THE EXTERIOR PORTIONS OF THE BRIDGE FOR SPANS 1 AND 2.
- CONSTRUCT THE BRIDGE DECK FOR THE EXTERIOR PORTIONS OF THE BRIDGE. REFER TO SLAB PLAN FOR SEQUENCE.
- MOVE TRAFFIC FROM THE EXISTING I.R. 75 BRIDGE TO THE NEWLY FINISHED PORTIONS OF THE BRIDGE.

PHASE II CONSTRUCTION:

- REMOVE STAGE 2 AND BUILD STAGE 3 OF THE TEMPORARY CONSTRUCTION ACCESS AS SHOWN ON SHEET [11/55] TO DEMOLISH AND CONSTRUCT THE INTERIOR PORTION OF THE I.R. 75 BRIDGE IN SPANS 1 AND 2.
- CONSTRUCT THE INTERIOR PORTION OF THE SUBSTRUCTURE FOR PIERS 1 AND 2 AND THE REAR ABUTMENT.
- SET THE SUPERSTRUCTURE BEAMS FOR THE INTERIOR PORTION OF THE BRIDGE FOR SPANS 1 AND 2.
- REMOVE STAGE 3 AND BUILD STAGE 4 OF THE TEMPORARY CONSTRUCTION ACCESS AS SHOWN ON SHEET [12/55] TO DEMOLISH AND CONSTRUCT THE INTERIOR PORTION OF THE I.R. 75 BRIDGE IN SPANS 3 THROUGH 5.
- CONSTRUCT THE INTERIOR PORTION OF THE SUBSTRUCTURE FOR PIERS 3 AND 4 AND THE FORWARD ABUTMENT.
- SET THE SUPERSTRUCTURE BEAMS FOR THE INTERIOR PORTION OF THE BRIDGE FOR SPANS 3 THROUGH 5.
- CONSTRUCT THE BRIDGE DECK AND CLOSURE POURS FOR THE INTERIOR PORTION OF THE BRIDGE. REFER TO SLAB PLAN FOR SEQUENCE.
- REMOVE STAGE 4 TEMPORARY CONSTRUCTION ACCESS.
- OPEN NEW STRUCTURE TO TRAFFIC.

NOTES:

- FOR ADDITIONAL MAINTENANCE OF TRAFFIC STAGING DETAILS, SEE MAINTENANCE OF TRAFFIC SHEETS.
- FOR TEMPORARY CONSTRUCTION ACCESS STAGE CONSTRUCTION DETAILS AND NOTES, SEE SHEET [9/55] THROUGH [12/55].

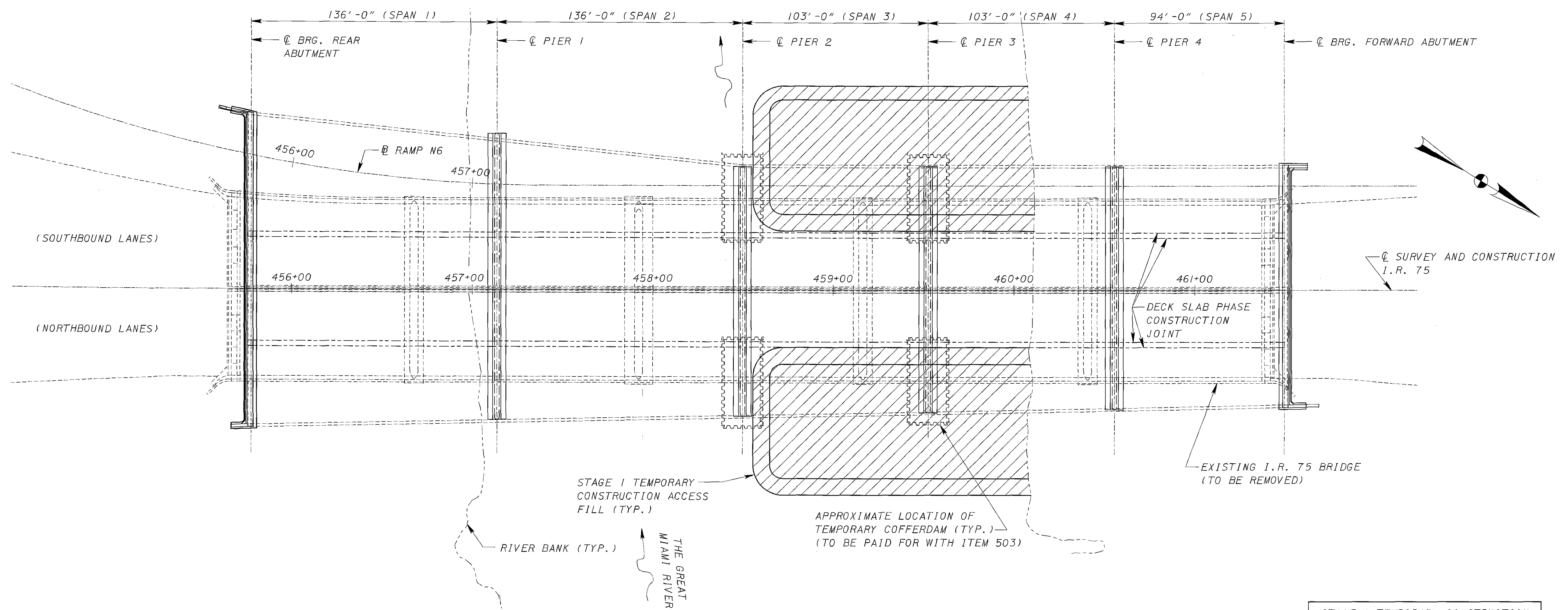
LEGEND:

- PHASE I CONSTRUCTION
- PHASE II CONSTRUCTION

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DESIGN AGENCY TransSystems 5000 N. W. 11th St., Suite 100 Ft. Lauderdale, FL 33309
REVIEWED DATE: 05/26/05 REVISION NUMBER: 5708710 DRAWN: JLV CHECKED: JGH DESIGNED: JGH
BRIDGE PHASE CONSTRUCTION AND NOTES BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION) I.R. 75 OVER THE GREAT MIAMI RIVER
MOT-75-14.60 PID 23828
8 / 55
243 314

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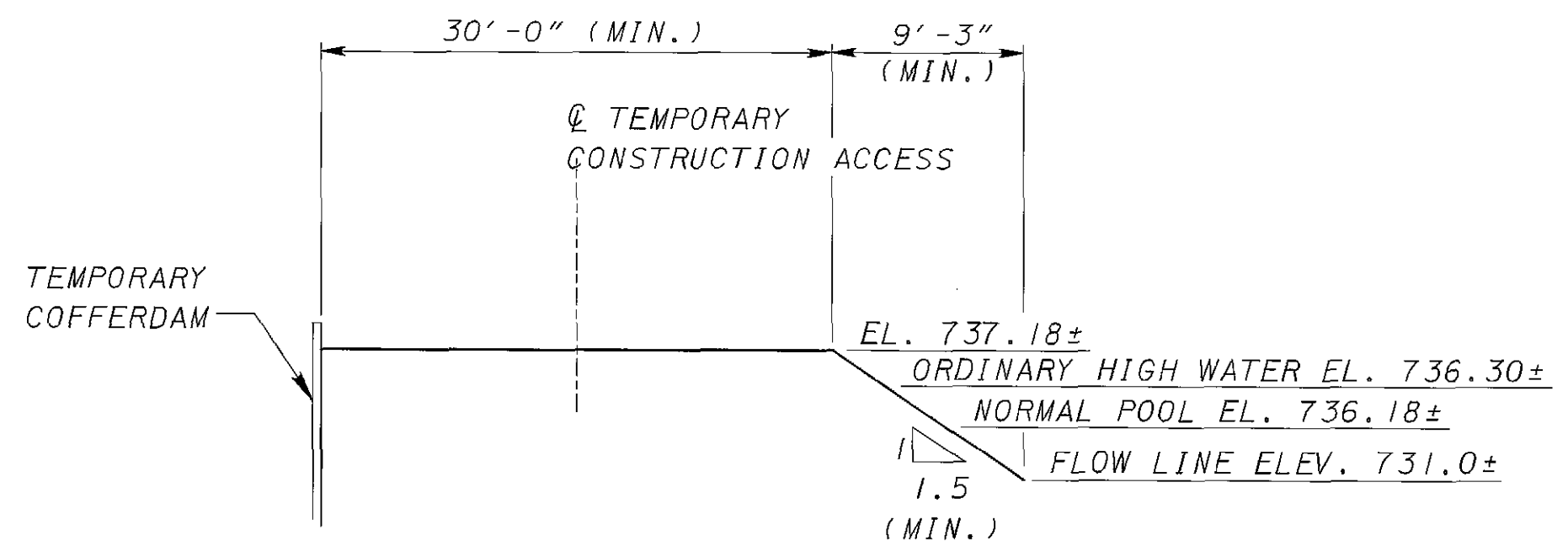


TEMPORARY CONSTRUCTION ACCESS - STAGE I PLAN

STAGE I TEMPORARY CONSTRUCTION
 ACCESS FILL
 0.57 ACRES
 4,900 CU. YD.

NOTES:

1. FOR ADDITIONAL MAINTENANCE OF TRAFFIC STAGING DETAILS, SEE MAINTENANCE OF TRAFFIC SHEETS.
2. TEMPORARY CONSTRUCTION ACCESS SHOWN FOR REMOVAL OF EXISTING STRUCTURE AND CONSTRUCTION OF PROPOSED STRUCTURE. ANY MODIFICATIONS TO CONSTRUCTION ACCESS SHALL BE DIRECTED TO ODOT OFFICE OF ENVIRONMENTAL SERVICES FOR REVIEW. NO DELAYS WILL BE GRANTED FOR ANY REASON WITH RESPECT TO CONTRACTOR MODIFICATIONS TO CONSTRUCTION ACCESS.
3. THE PLACEMENT OF THE TEMPORARY CONSTRUCTION ACCESS MATERIALS SHALL BE PERFORMED BETWEEN JUNE 30 AND APRIL 15 OF THE FOLLOWING YEAR. CONSTRUCTION ACCESSES WHICH HAVE BEEN CONSTRUCTED TO THE ELEVATION SHOWN ON THE PLANS PRIOR TO APRIL 15 CAN BE ACCESSED AT ALL TIMES AND SHALL NOT BE CONSIDERED WORK WITHIN THE WATERWAY.
4. WORK WITHIN ANY CLOSED CELL COFFERDAMS SHALL NOT BE CONSIDERED WORK WITHIN THE RIVER DURING THE RESTRICTED DATES OF APRIL 15 THROUGH JUNE 30 AND SHALL BE SUBJECT TO THE DEWATERING SPECIFICATIONS OF CMS 107 AT ALL TIMES.
5. THE CONSTRUCTION ACCESSES SHALL BE CONSTRUCTED TO ODOT SUPPLEMENTAL SPECIFICATION 832 UNLESS OTHERWISE SPECIFIED ON THE PLANS AND STANDARD DRAWINGS.
6. AT NO TIME SHALL THE TEMPORARY CONSTRUCTION ACCESS AND COFFERDAMS BE BUILT TO IMPEDE NORMAL WATER FLOW.
7. TEMPORARY CULVERTS SHALL NOT BE REQUIRED. HOWEVER, NORMAL RIVER FLOW SHALL BE MAINTAINED AT ALL TIMES.
8. ALL TEMPORARY CONSTRUCTION ACCESS FILL MATERIAL SHALL BE REMOVED FROM THE RIVER AT THE COMPLETION OF THE PROJECT, AND THE RIVER BOTTOM RESTORED TO ITS ORIGINAL ELEVATIONS.
9. SEE PROJECT SPECIAL PROVISIONS FOR 404/401 PERMITS.
10. FOR PHASE CONSTRUCTION SEQUENCE NOTES, SEE SHEET **8155**.
11. ALL COSTS ASSOCIATED WITH CONSTRUCTING THE TEMPORARY CONSTRUCTION ACCESS FILLS SHALL BE PAID FOR BY THE CONTRACTOR IN ACCORDANCE WITH SUPPLEMENTAL SPECIFICATION 832.10.

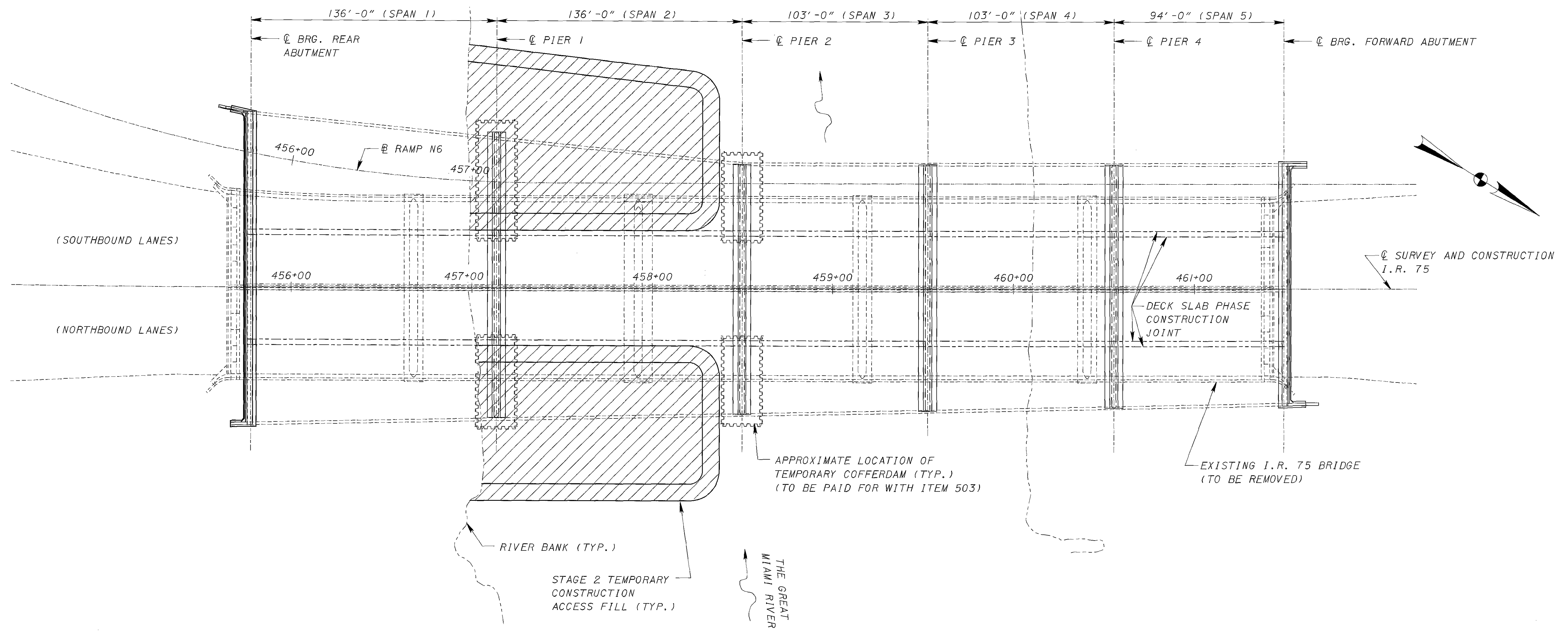


TYPICAL STAGE I CONSTRUCTION ACCESS SECTION ADJACENT TO COFFERDAM

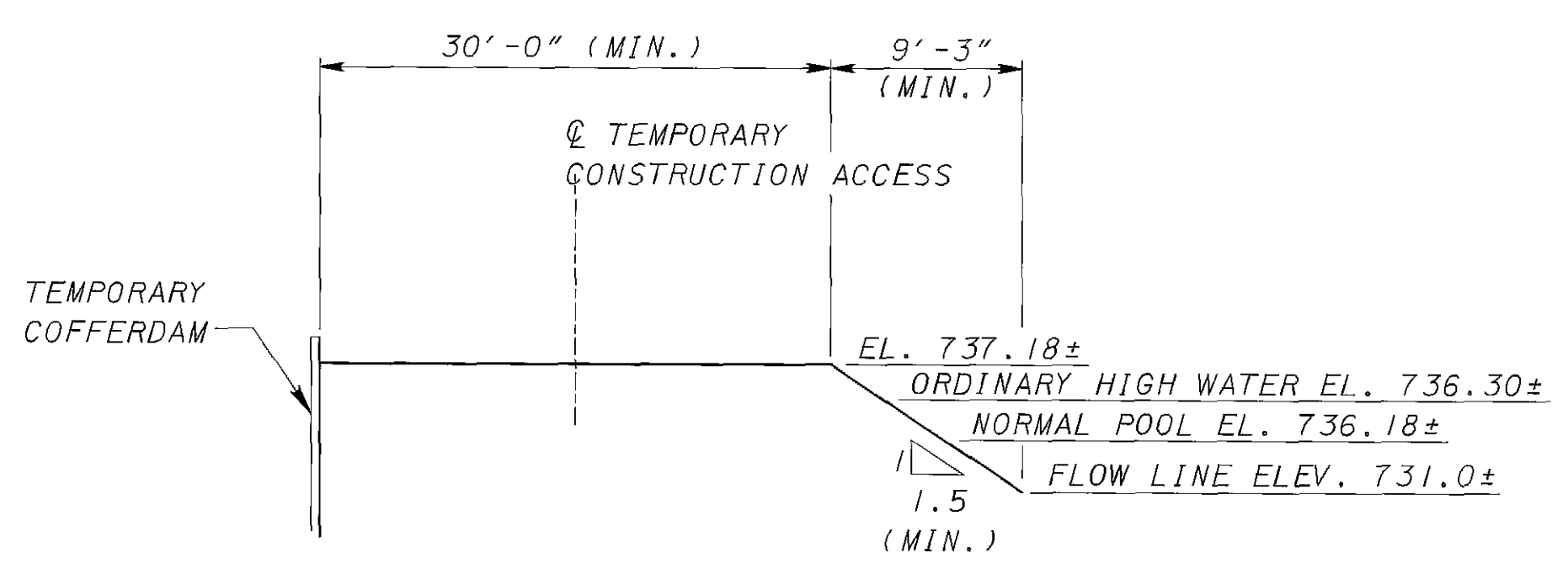
LEGEND:

TEMPORARY CONSTRUCTION ACCESS FILL

PLOTTED BY: ecmack DATE: 7/27/2007 TIME: 3:41:53 PM
 File name: g:\co04\0066\brldge\concr\ete.dwg\cadd\23828m05.dgn



TEMPORARY CONSTRUCTION ACCESS - STAGE 2 PLAN



TYPICAL STAGE 2 CONSTRUCTION ACCESS SECTION ADJACENT TO COFFERDAM

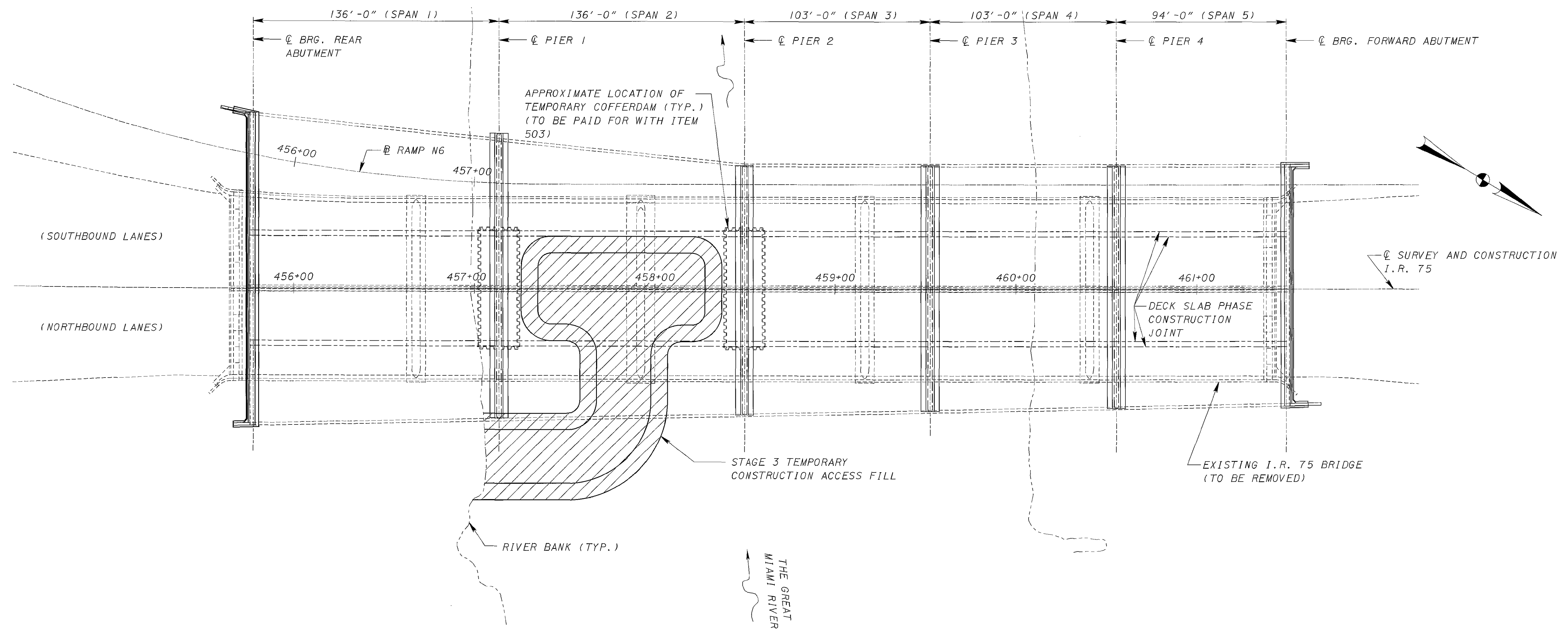
LEGEND:
 TEMPORARY CONSTRUCTION ACCESS FILL

STAGE 2 TEMPORARY CONSTRUCTION ACCESS FILL
 0.56 ACRES
 4,875 CU.YD.

NOTE:
 1. FOR NOTES, SEE TEMPORARY CONSTRUCTION ACCESS - STAGE 1 SHEET.

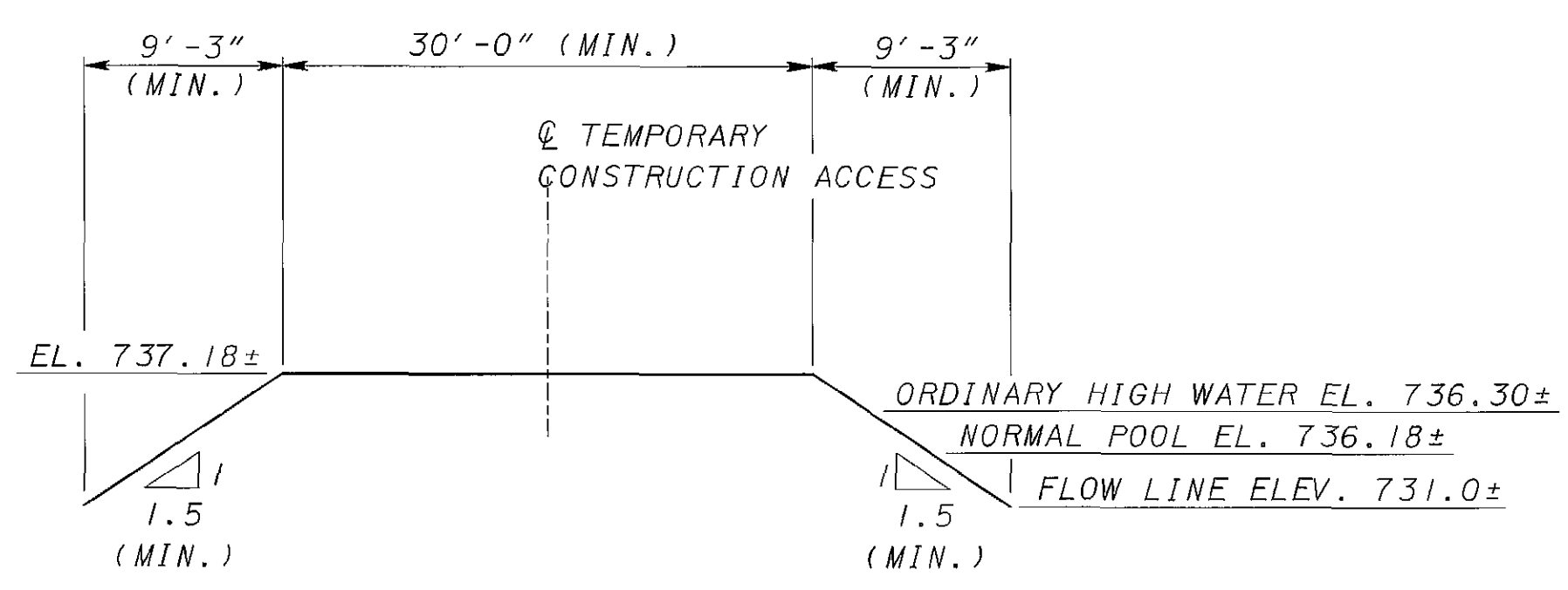
DESIGN AGENCY TransSystems <small>55 PUBLIC SQUARE BRIDGE DRIVE, 4TH FLOOR CLEVELAND, OHIO 44115-4800</small>	DATE 05/26/05
	STRUCTURE FILE NUMBER 5708710
REVIEWED RER	DRAWN JLV
DESIGNED GHD	CHECKED JDH
TEMPORARY CONSTRUCTION ACCESS - STAGE 2 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION) I.R. 75 OVER THE GREAT MIAMI RIVER	
MOT-75-14.60 PID 23828	
10/55	
245 314	

PLOTTED BY: ecmack DATE: 7/27/2007 TIME: 3:44:58 PM
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TEMPORARY CONSTRUCTION ACCESS - STAGE 3 PLAN

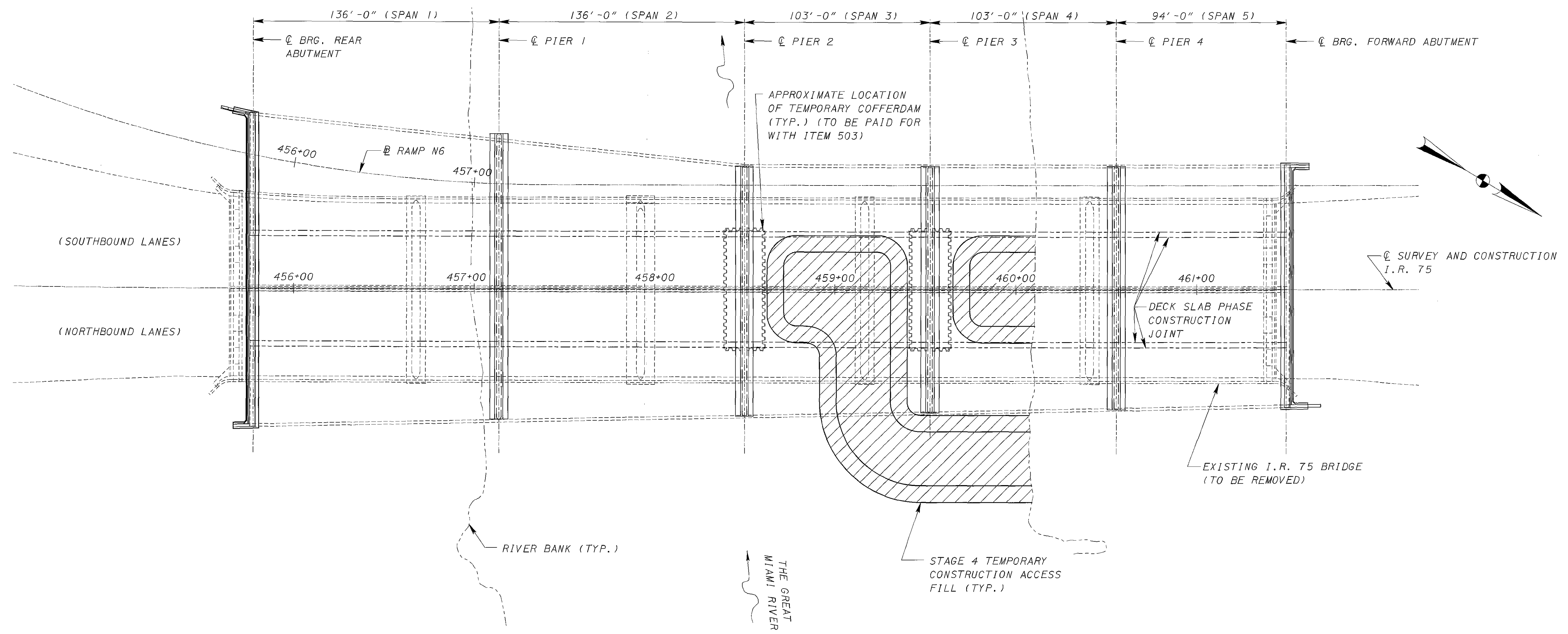
STAGE 3 TEMPORARY CONSTRUCTION
 ACCESS FILL
 0.29 ACRES
 2,325 CU. YD.



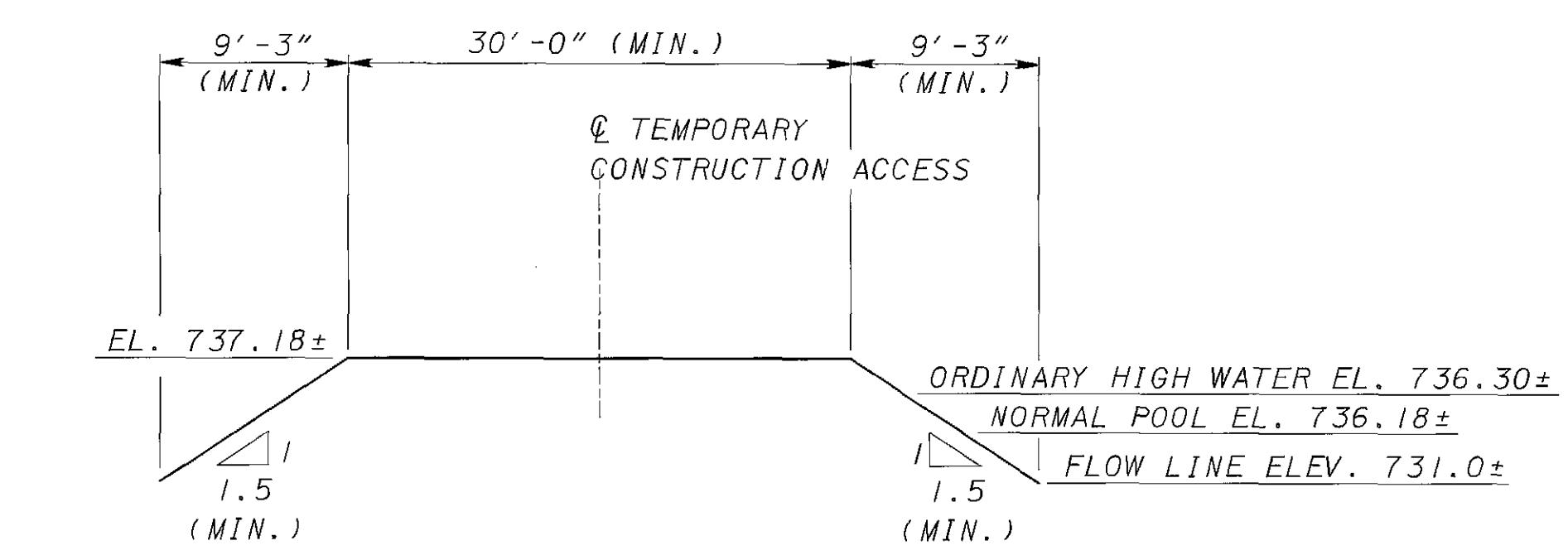
TYPICAL STAGE 3 CONSTRUCTION ACCESS SECTION

LEGEND:
 TEMPORARY CONSTRUCTION ACCESS FILL

NOTE:
 1. FOR NOTES, SEE TEMPORARY CONSTRUCTION ACCESS - STAGE 1 SHEET.



TEMPORARY CONSTRUCTION ACCESS - STAGE 4 PLAN



TYPICAL STAGE 4 CONSTRUCTION ACCESS SECTION

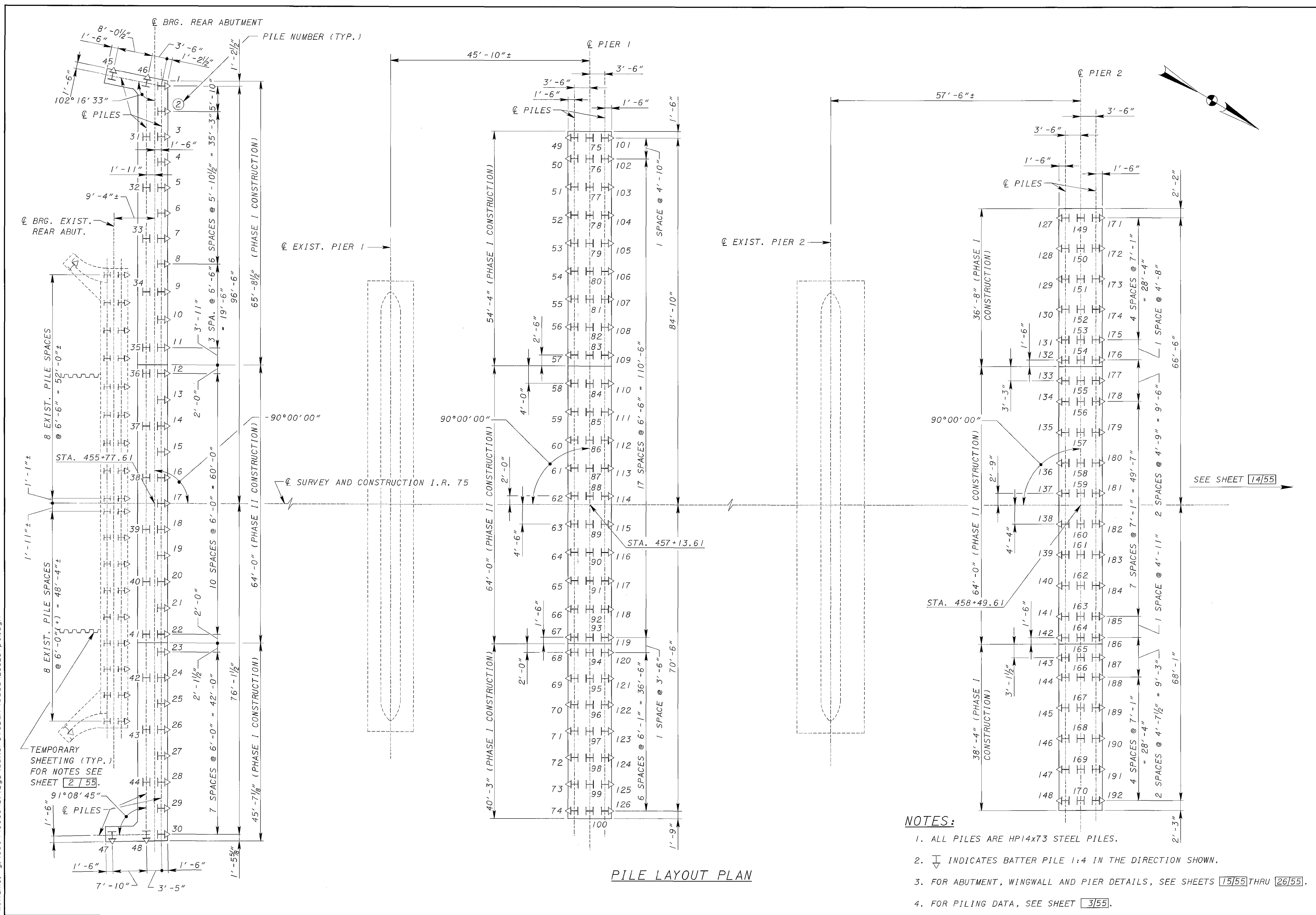
LEGEND:

TEMPORARY CONSTRUCTION ACCESS FILL

STAGE 4 TEMPORARY CONSTRUCTION ACCESS FILL
 0.32 ACRES
 2,565 CU. YD.

NOTE:
 1. FOR NOTES, SEE TEMPORARY CONSTRUCTION ACCESS - STAGE 1 SHEET.

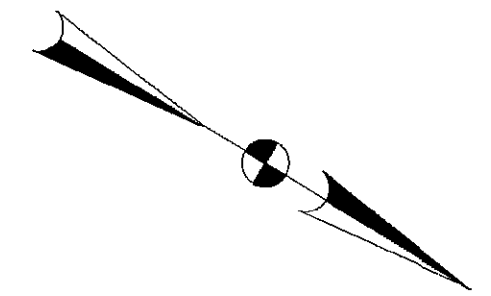
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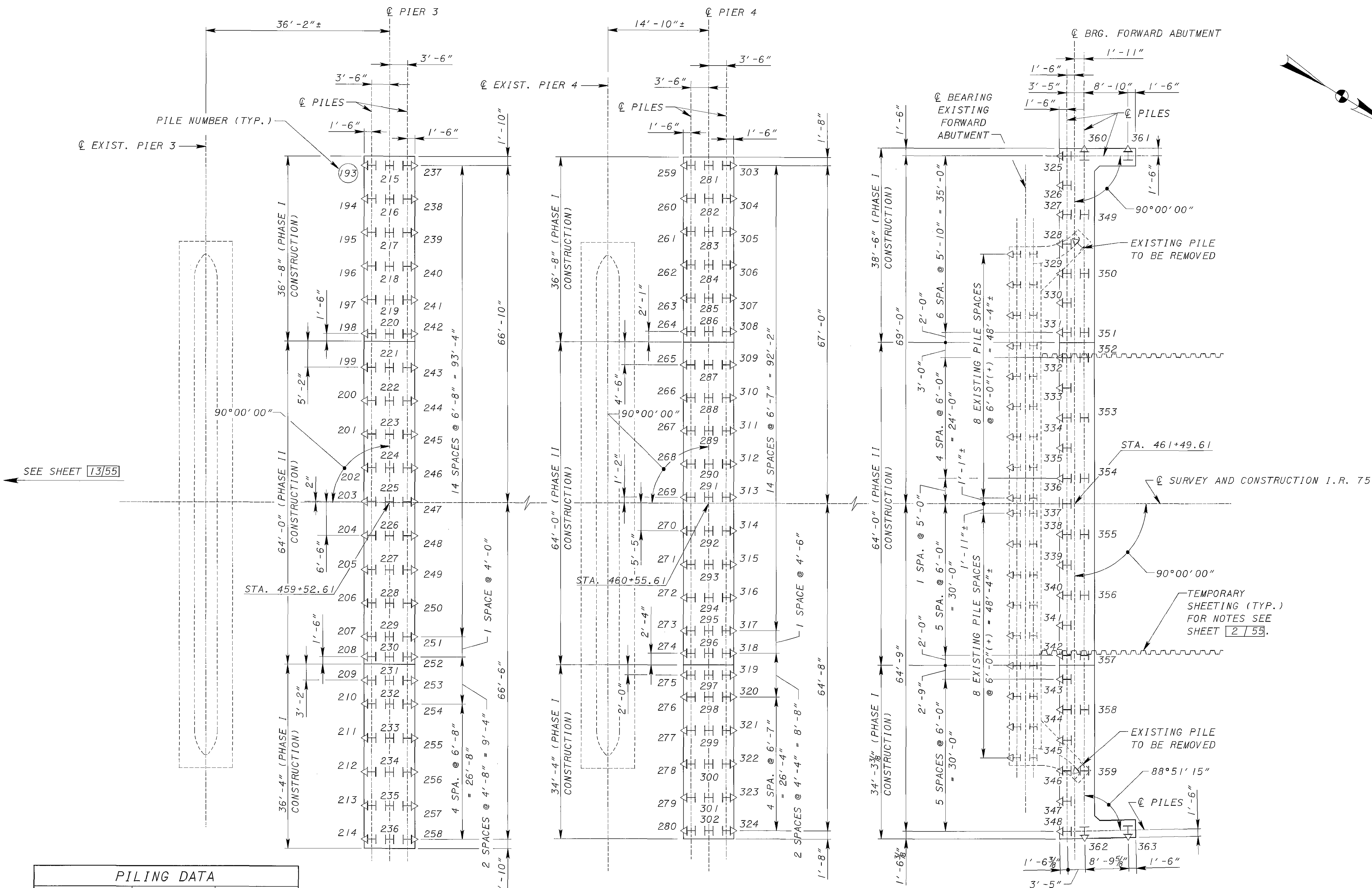
PILE LAYOUT PLAN

NOTES:

1. ALL PILES ARE HPI4x73 STEEL PILES.
2. ▽ INDICATES BATTER PILE 1:4 IN THE DIRECTION SHOWN.
3. FOR ABUTMENT, WINGWALL AND PIER DETAILS, SEE SHEETS [15/55] THRU [26/55].
4. FOR PILING DATA, SEE SHEET [3/55].



DESIGNED JDH CHECKED MFF	DATE 05/26/05 STRUCTURE FILE NUMBER 5708710
PILE LAYOUT PLAN (PRESTRESSED CONCRETE OPTION)	
BRIDGE NO. MOT-75-1523 I.R-75 OVER THE GREAT MIAMI RIVER	
MOT-75-14.60 PID 23828	
13/55	
248 314	



SEE SHEET 13/55

PILING DATA		
SUBSTRUCTURE UNIT	PILE CUTOFF ELEVATION *	PILE TIP ELEVATION
REAR ABUT.	751.75	-
PIER 1	730.50	708.00 **
PIER 2	725.50	704.00 **
PIER 3	725.50	708.00 **
PIER 4	730.50	715.00 **
FORWARD ABUT.	757.50	-

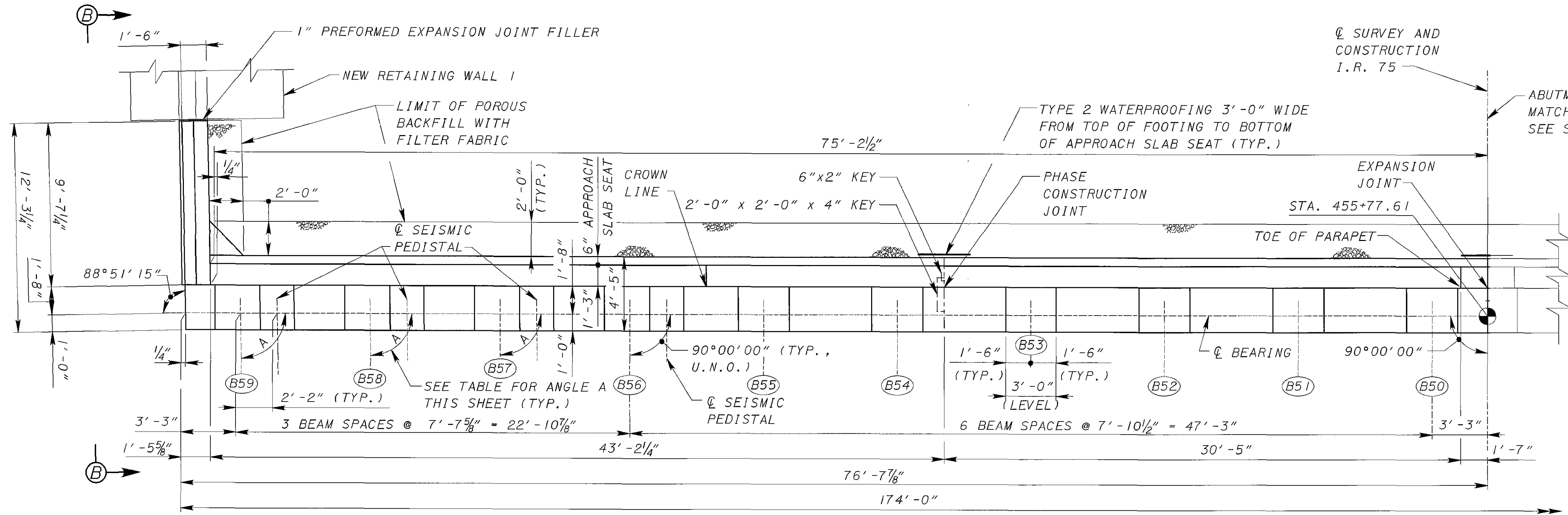
* - PILE CUTOFF ELEVATION EQUALS BOTTOM OF FOOTING ELEVATION + 1'-0" FOR ABUTMENTS, AND BOTTOM OF FOOTING ELEVATION + 1'-6" FOR PIERS.
 ** - ALL PIER PILES MUST BE DRIVEN TO THE PILE TIP ELEVATION SHOWN DUE TO SCOUR DEPTH.

PILE LAYOUT PLAN

NOTES:

- FOR NOTES, SEE SHEET 13/55.
- THE REMOVAL OF 2-EXISTING PILES AT THE FORWARD ABUTMENT SHALL BE INCLUDED WITH ITEM 202 - PORTIONS OF STRUCTURES REMOVED, OVER 20 FOOT SPAN, AS PER PLAN

Plotted By: emack Date: 7/27/2007 Time: 3:41:33 PM
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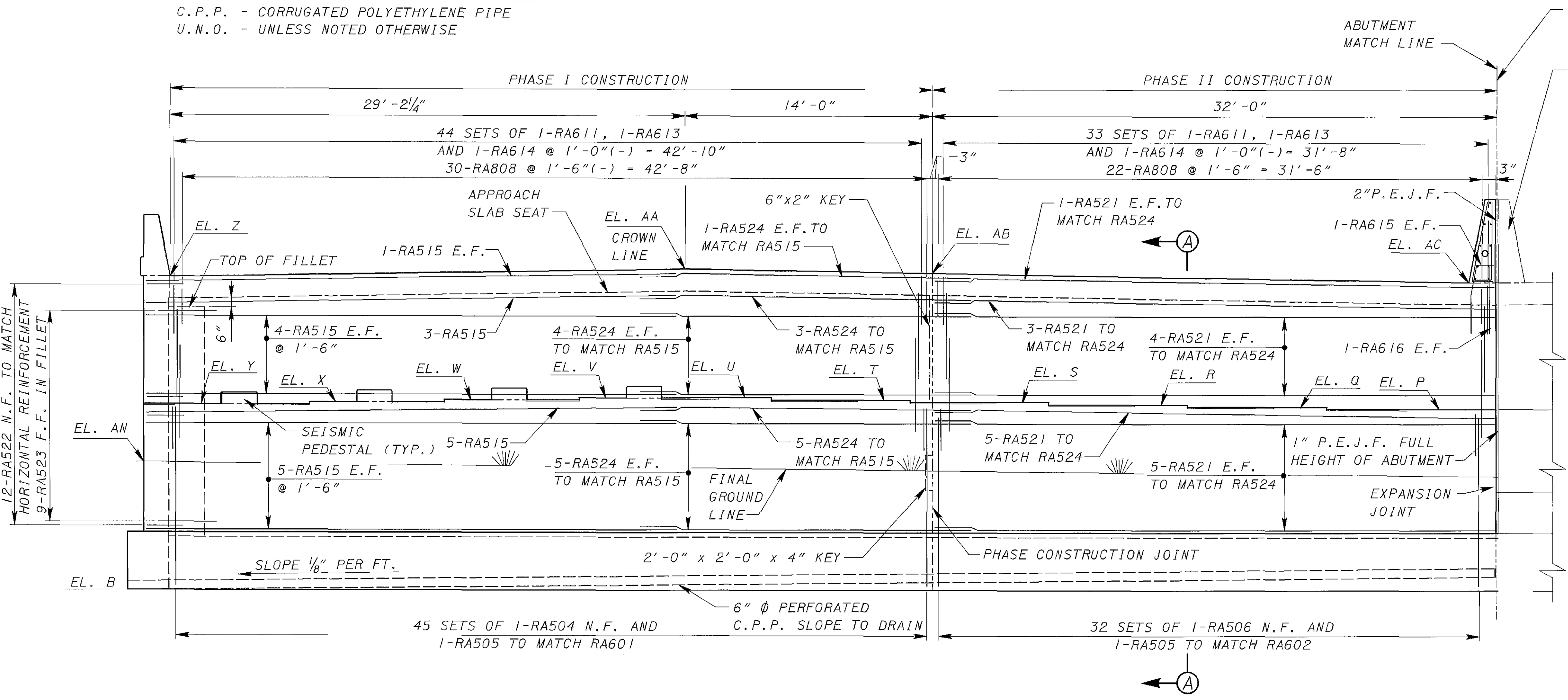
LEGEND:

- (BX) = BEAM NUMBER
- E.F. - EACH FACE
- F.F. - FAR FACE
- N.F. - NEAR FACE
- P.E.J.F. - PREFORMED EXPANSION JOINT FILLER
- C.P.P. - CORRUGATED POLYETHYLENE PIPE
- U.N.O. - UNLESS NOTED OTHERWISE

PART-PLAN

TABLE OF BEAM ANGLES	
BEAM	ANGLE A
(B57)	89° 37' 07"
(B58)	89° 14' 15"
(B59)	88° 51' 15"

TABLE OF ELEVATIONS	
LOCATION	ELEVATION
B	750.75
P	761.00
Q	761.13
R	761.25
S	761.38
T	761.51
U	761.64
V	761.62
W	761.50
X	761.37
Y	761.25
Z	768.53
AA	769.00
AB	768.77
AC	768.27
AN	758.25



PART-ELEVATION
(PILES NOT SHOWN)

FOR MEDIAN BARRIER DETAIL SEE APPROACH SLAB PLAN.
 PAYMENT FOR MEDIAN BARRIER SHALL BE INCLUDED WITH
 ITEM 898 - QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE
 (APPROACH SLAB), 15", AS PER PLAN.

NOTES:

1. POROUS BACKFILL WITH FILTER FABRIC, 2 FEET THICK SHALL EXTEND UP TO THE PLANE OF THE SUBGRADE, TO 1 FOOT BELOW THE EMBANKMENT SURFACE, AND Laterally TO THE ENDS OF THE WINGWALLS.
2. BACKWALL CONCRETE: IN ADDITION TO 511.10, DO NOT PLACE BACKWALL CONCRETE ABOVE THE OPTIONAL CONSTRUCTION JOINT AT THE APPROACH SLAB SEAT UNTIL AFTER THE DECK CONCRETE IN THE SPAN ADJACENT TO THE ABUTMENT HAS BEEN PLACED.
3. SEALING OF BEAM SEATS: IF THE BEAMS SEATS ARE SEALED WITH AN EPOXY OR NON-EPOXY SEALER PRIOR TO SETTING THE BEARINGS, DO NOT APPLY SEALER TO THE CONCRETE SURFACES UNDER THE PROPOSED BEARING LOCATIONS. IF THESE LOCATIONS ARE SEALED, REMOVE THE SEALER TO THE SATISFACTION OF THE ENGINEER PRIOR TO SETTING THE BEARINGS. THE DEPARTMENT WILL NOT PAY FOR THIS REMOVAL.
4. FOR FOOTING PLAN AND SECTION A-A, SEE SHEET [17]55.
5. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [17]55.
6. FOR ELEVATION B-B, SEE SHEET [18]55.
7. FOR BEARING DEVICES, SEE SHEET [49]55.
8. FOR APPROACH SLAB DETAILS, SEE SHEET [51]55.
9. FOR REINFORCING STEEL LIST, SEE SHEET [52]55.
10. FOR LAP LENGTH TABLE, SEE SHEET [17]55.
11. FOR EXPANSION JOINT DETAILS, SEE SHEET [50]55.

DESIGN AGENCY: **TranSystems**
 9600 W. 15th Ave., Suite 400
 Denver, CO 80202
 PHONE: 303.751.5890

DATE: 05/26/05
 REVIEWED: NFF
 STRUCTURE FILE NUMBER: 5708710

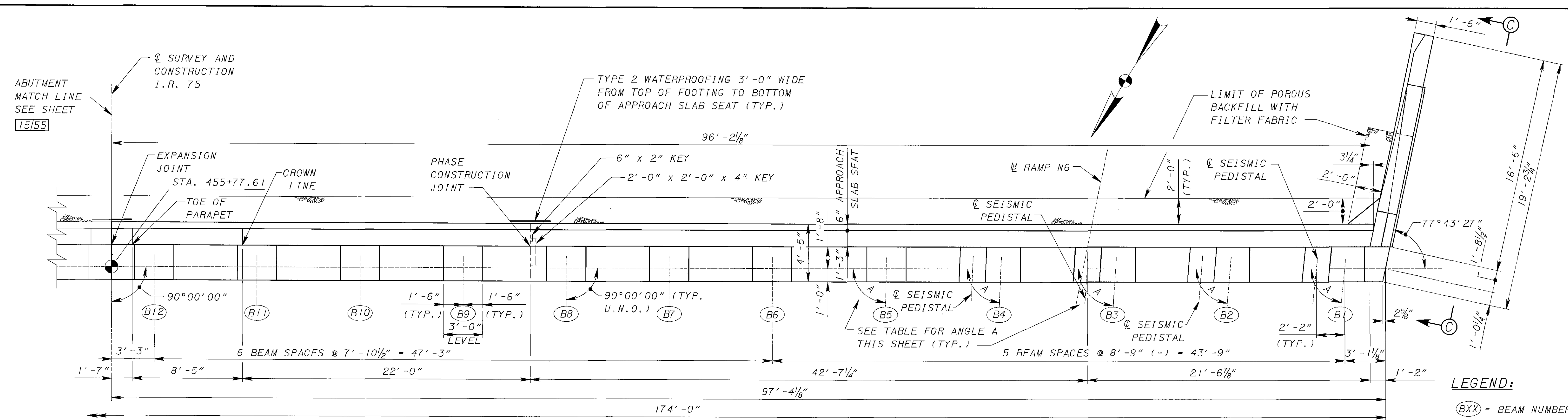
DRAWN: RCK
 CHECKED: JDH

DESIGNED: BTA
 CHECKED: JDH

REAR ABUTMENT PLAN AND ELEVATION
 BRIDGE NO. MOT-75-1523
 I.R. 75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

15/55
 250
 314

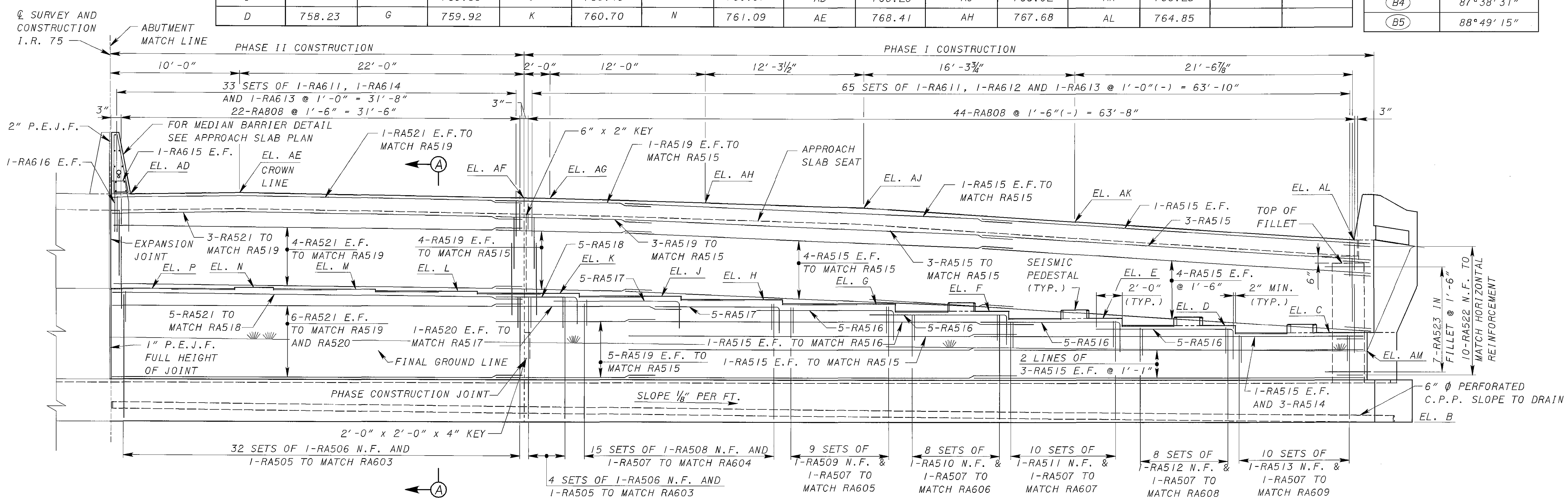


PART-PLAN

TABLE OF ELEVATIONS

LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION	LOCATION	ELEVATION
B	750.75	E	758.79	H	760.24	L	760.84	P	761.00	AF	768.05	AJ	767.31	AM	756.25
C	757.66	F	759.36	J	760.48	M	760.97	AD	768.28	AG	768.02	AK	766.25		
D	758.23	G	759.92	K	760.70	N	761.09	AE	768.41	AH	767.68	AL	764.85		

BEAM	ANGLE A
(B1)	84°07'17"
(B2)	85°17'28"
(B3)	86°27'54"
(B4)	87°38'31"
(B5)	88°49'15"

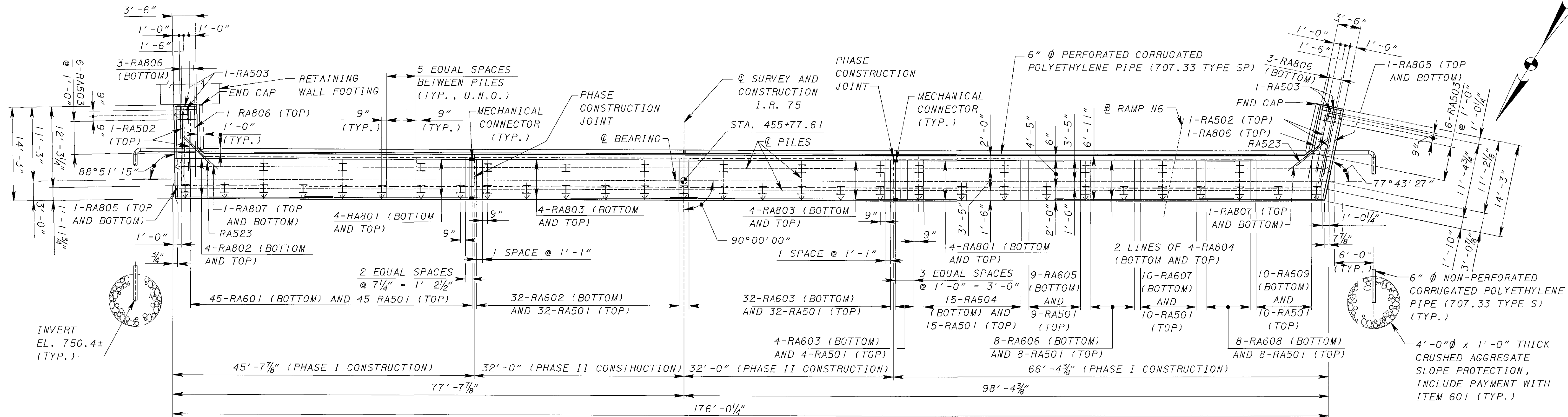


PART-ELEVATION
 (PILES NOT SHOWN)

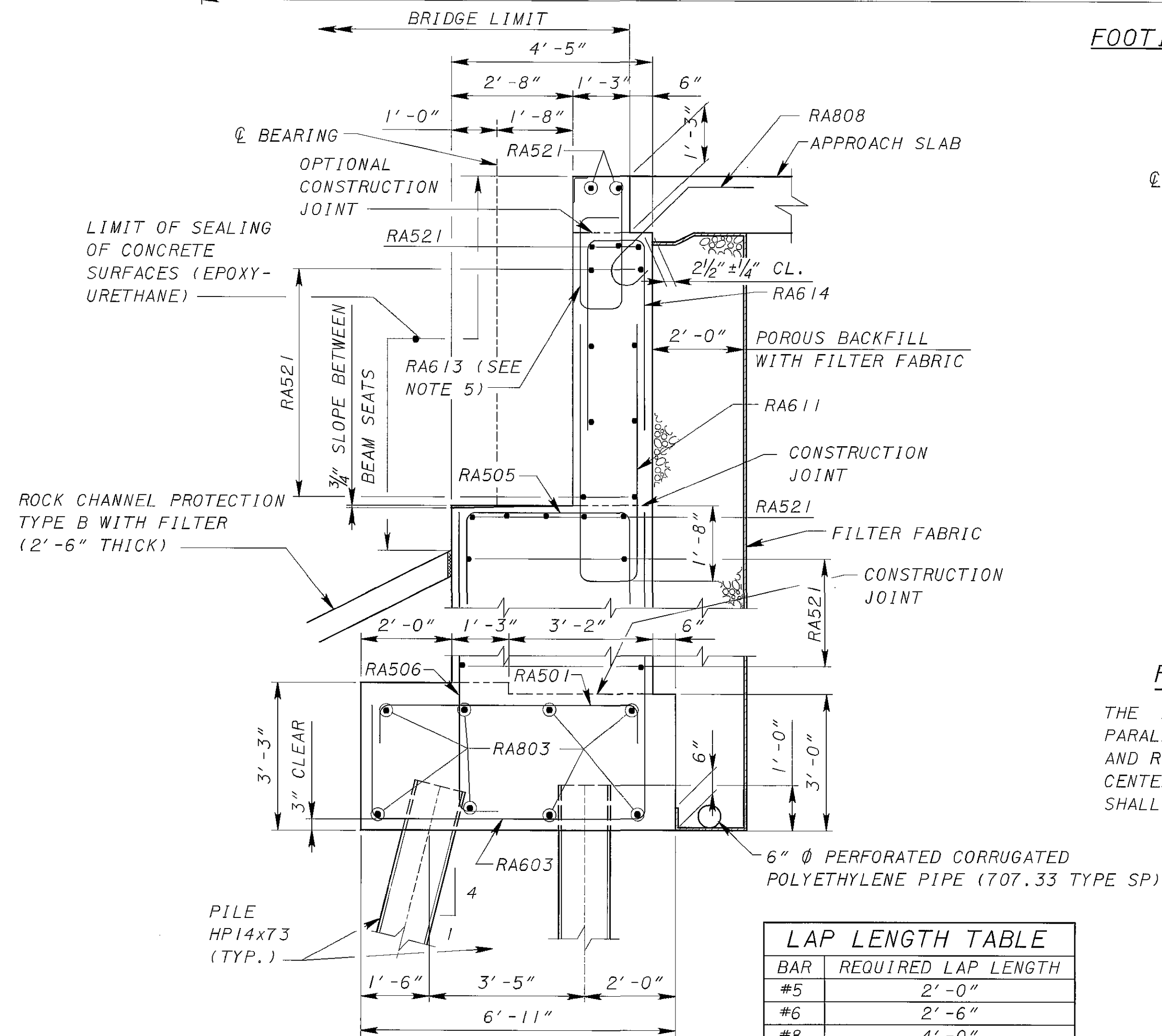
- NOTES:**
- FOR ADDITIONAL NOTES SEE SHEET 15/55.
 - FOR ELEVATION C-C SEE SHEET 18/55.

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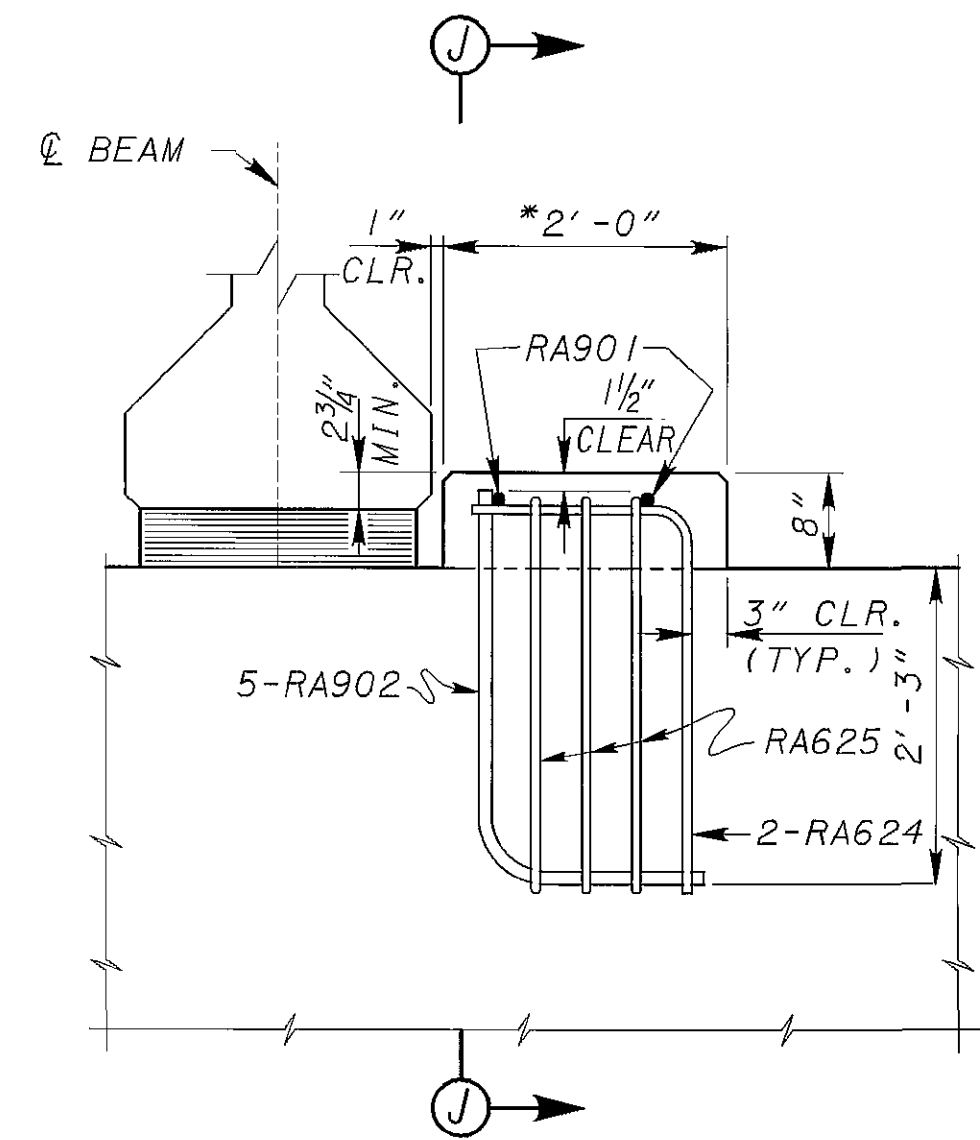
FOOTING PLAN



SECTION A-A

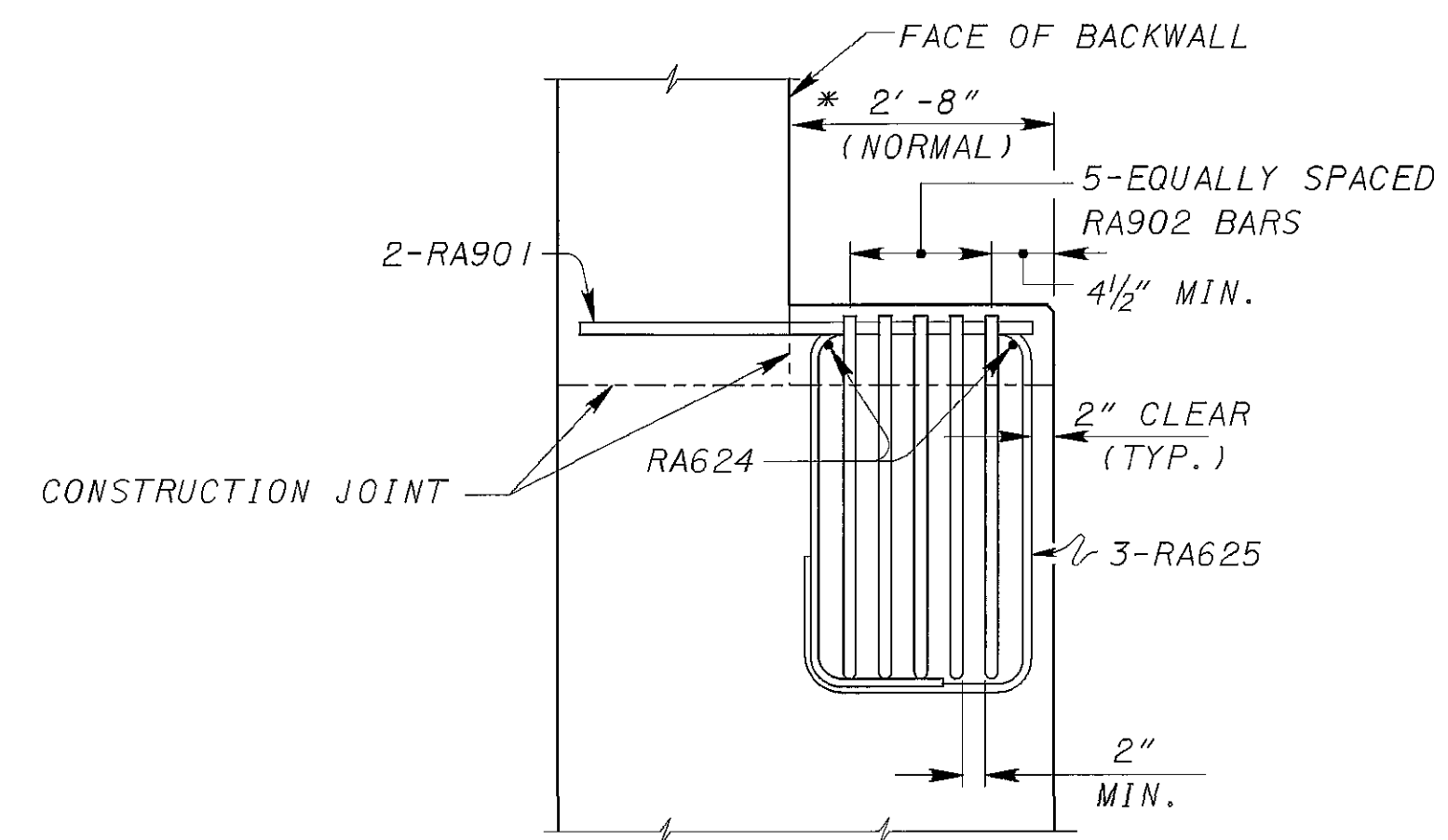
(EXPANSION JOINT ANCHOR NOT SHOWN)

LAP LENGTH TABLE	
BAR	REQUIRED LAP LENGTH
#5	2'-0"
#6	2'-6"
#8	4'-0"



FRONT VIEW OF SEISMIC PEDESTAL

THE 2'-0" WIDTH OF THE PEDESTAL SHALL BE MEASURED PARALLEL TO THE CENTERLINE OF BEARING. THE RA902 AND RA624 BARS SHALL BE PLACED PARALLEL TO THE CENTERLINE OF BEARING. THE RA901 AND RA625 BARS SHALL BE PLACED PARALLEL TO THE BEAMS OR GIRDERS.



SECTION J-J

THE LOCATION OF THE MAIN REINFORCEMENT IN THE BEAM SEAT MAY BE ADJUSTED HORIZONTALLY ±1" TO ACCOMMODATE THE RA902 BARS.

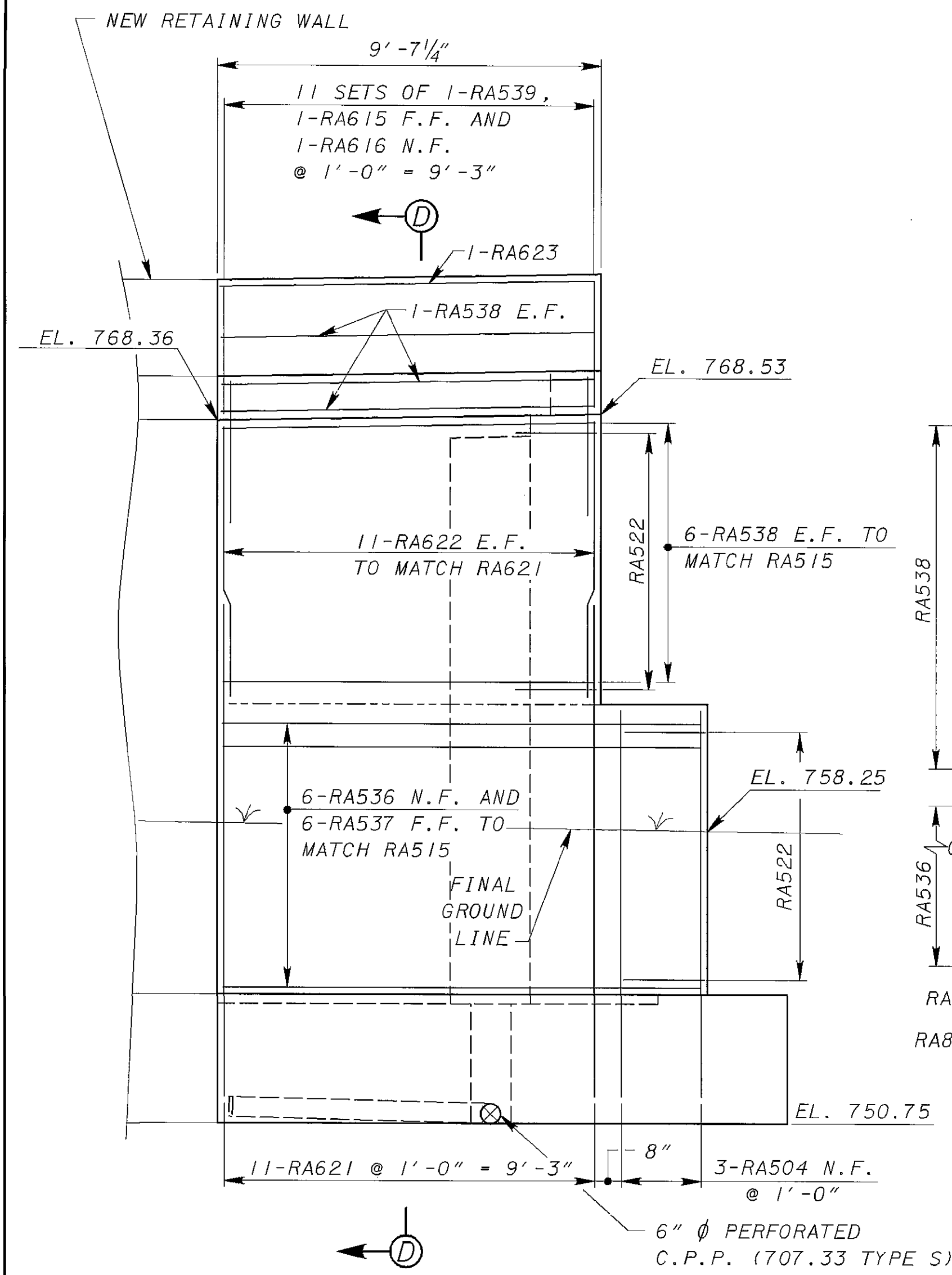
* - THE SURFACE OF THE BEAM SEAT IN THIS AREA SHALL BE FINISHED WITH A SERRATED TROWEL. THE SERRATIONS SHALL BE 1/4" DEEP MINIMUM.

NOTE:

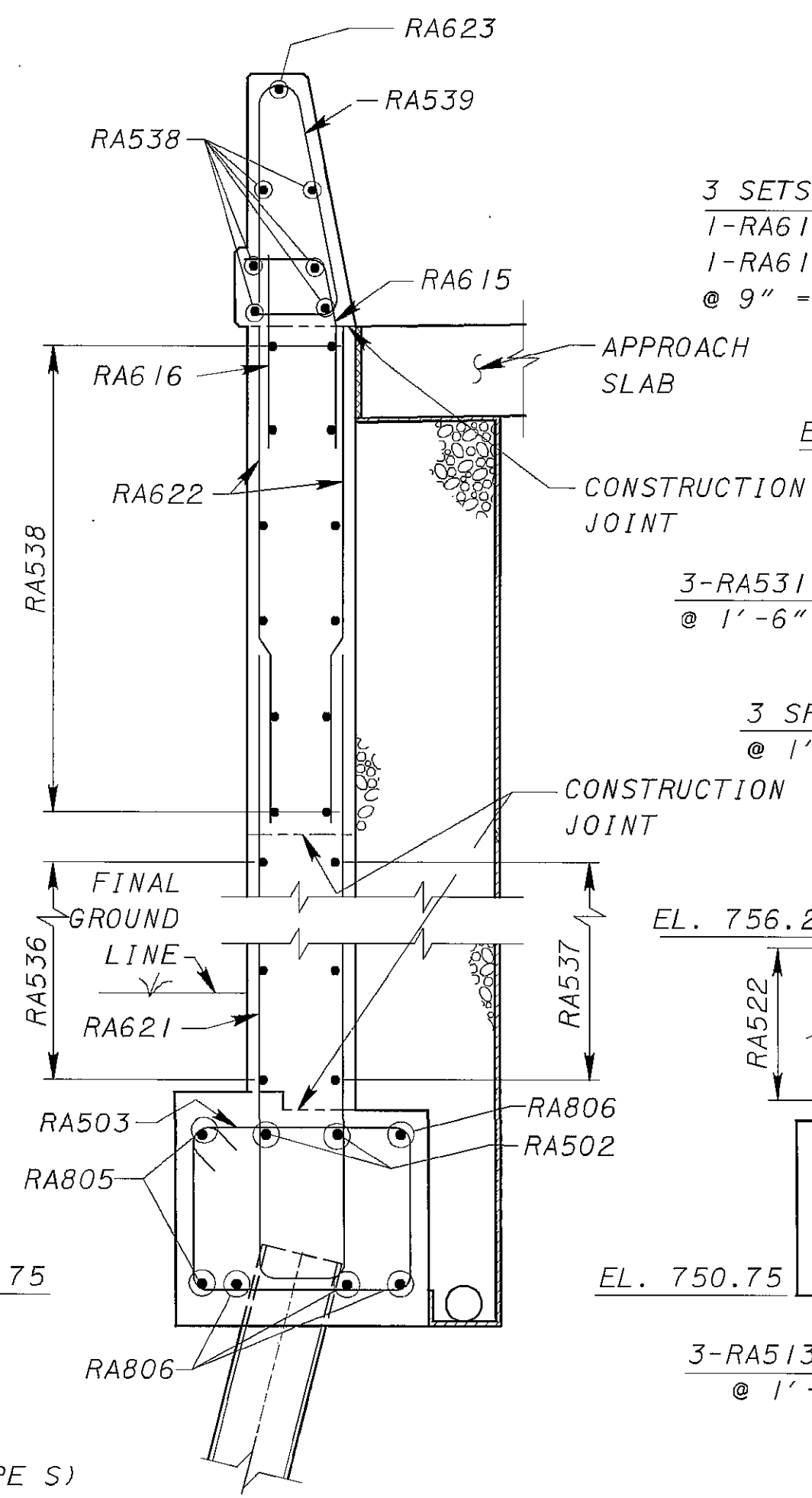
1. FOR ABUTMENT PLAN AND ELEVATION SEE SHEET [15/55].
2. FOR ADDITIONAL NOTES SEE SHEET [15/55].
3. FOR PILE LOCATIONS, SEE SHEETS [13/55] AND [14/55].
4. FOR LOCATIONS OF SEISMIC PEDESTALS, SEE SHEETS [15/55] AND [16/55].
5. FOR PLACEMENT OF RA613 BARS, SEE SHEET [50/55].

DESIGN AGENCY: **TranSystems**
 DATE: 05/26/05
 REVIEWED: NFF
 DRAWN: RCK
 DESIGNED: BTA
 CHECKED: JDH
 STRUCTURE FILE NUMBER: 5708710
 REAR ABUTMENT FOOTING PLAN AND DETAILS (PRESTRESSED CONCRETE OPTION)
 BRIDGE NO. MOT-75-1523 I.R. 75 OVER THE GREAT MIAMI RIVER
 MOT-75-14.60
 PID 23828
 17/55
 252
 314

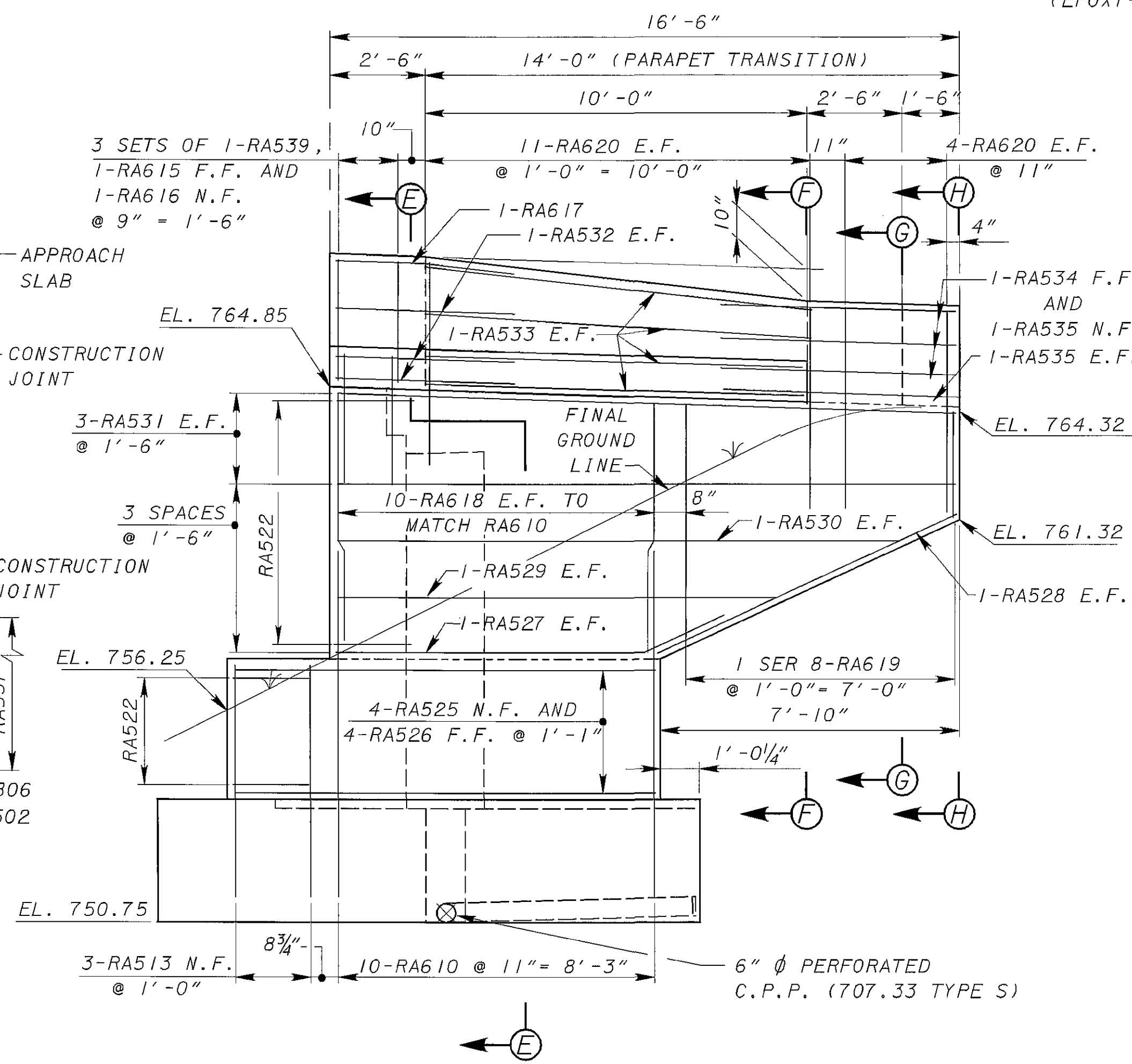
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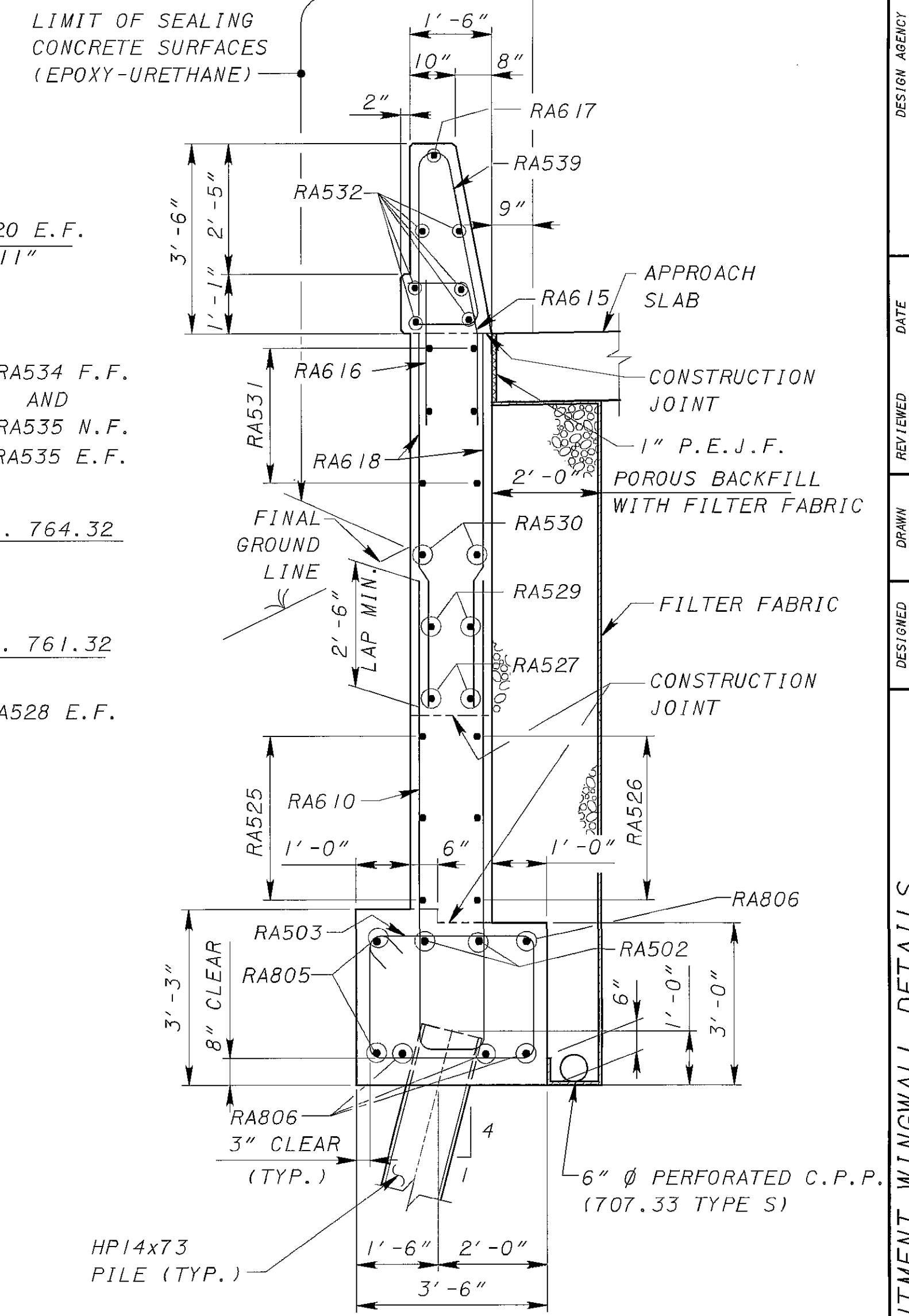
ELEVATION B-B
(PILES NOT SHOWN)



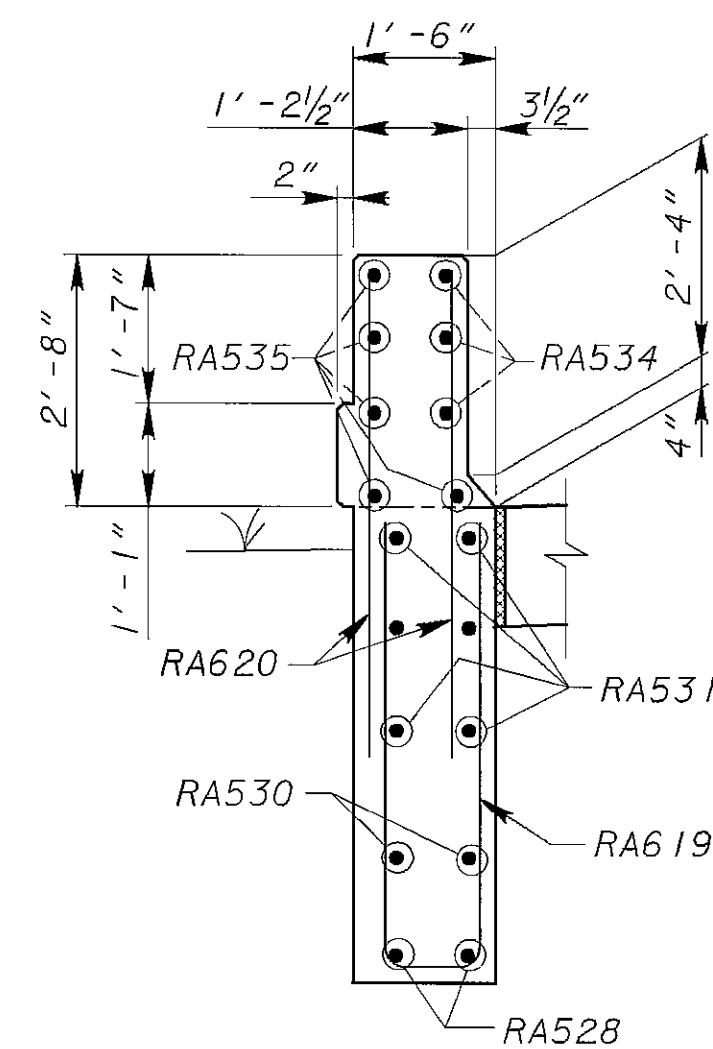
SECTION D-D
(FOR DETAILS NOT SHOWN
SEE SECTION E-E)



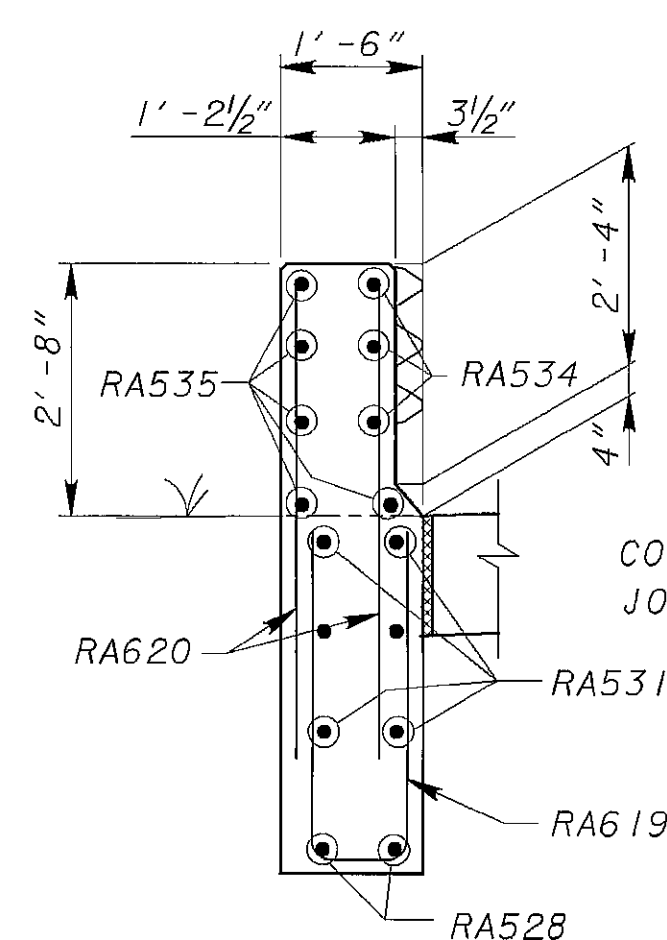
ELEVATION C-C
(PILES NOT SHOWN)



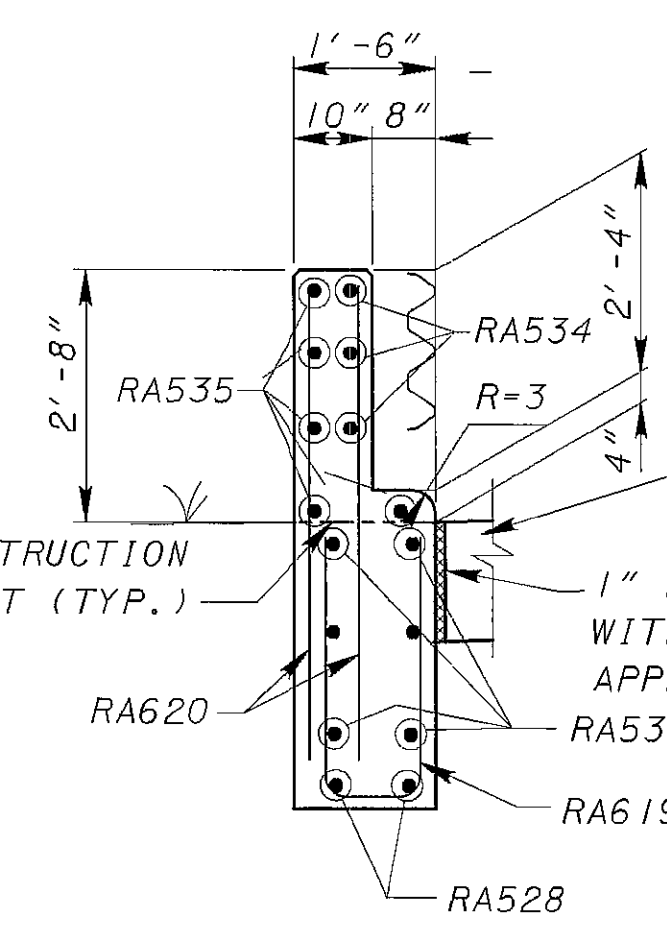
SECTION E-E



SECTION F-F



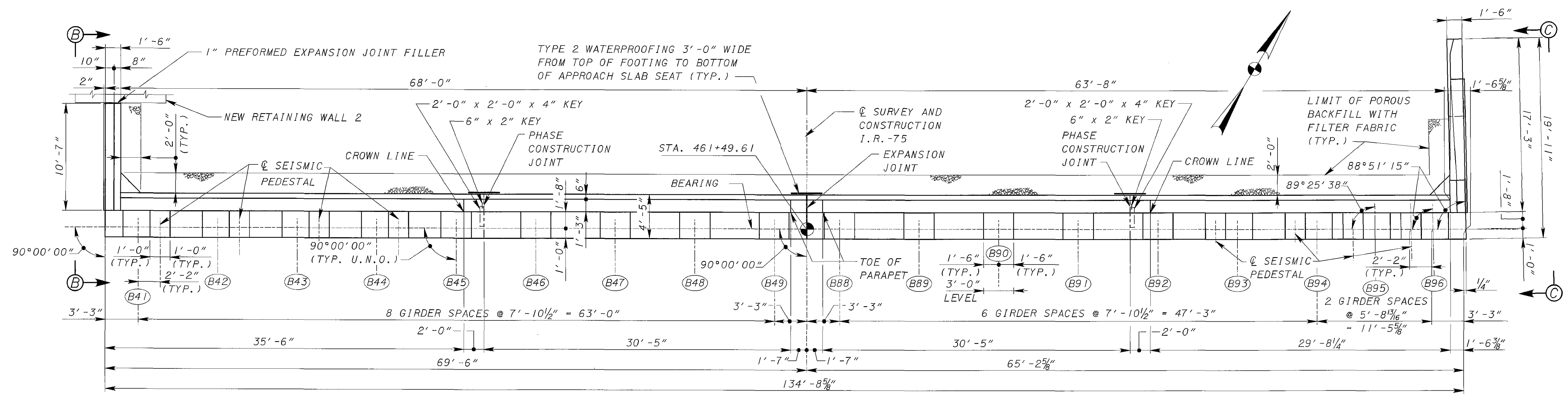
SECTION G-G



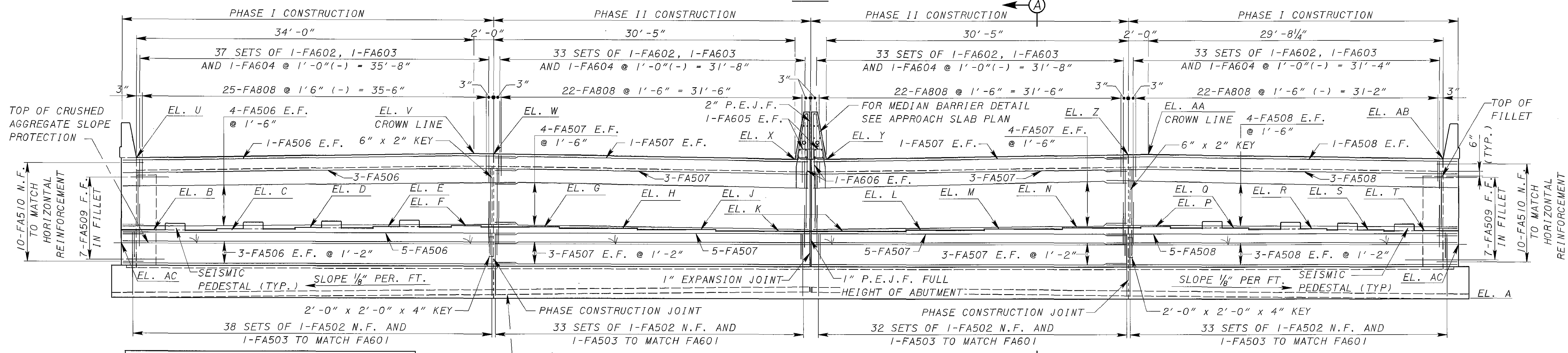
SECTION H-H

NOTES:

1. FOR ABUTMENT DETAILS, SEE SHEETS **15/55** AND **16/55**.
2. FOR ADDITIONAL NOTES, SEE SHEET **15/55**.



PLAN



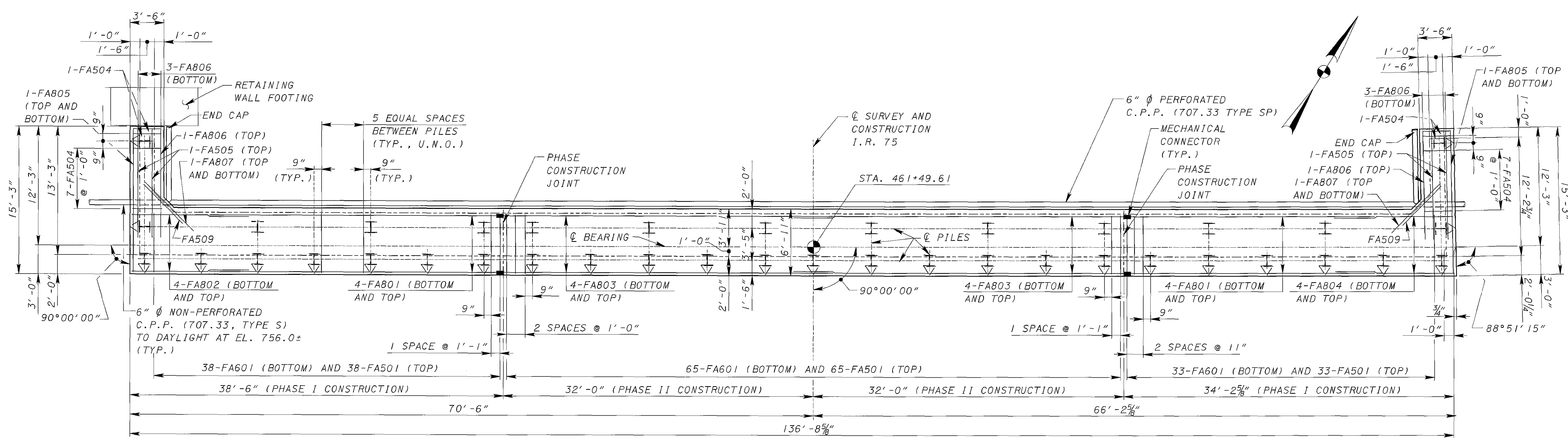
ELEVATION
(PILES NOT SHOWN)

ELEVATION TABLE			
LOCATION	ELEVATION	LOCATION	ELEVATION
A	756.50	P	763.87
B	763.37	Q	763.75
C	763.49	R	763.62
D	763.62	S	763.53
E	763.75	T	763.44
F	763.87	U	770.62
G	763.77	V	771.16
H	763.64	W	771.13
J	763.52	X	770.65
K	763.39	Y	770.65
L	763.52	Z	771.13
M	763.64	AA	771.16
N	763.77	AB	770.69
		AC	762.00

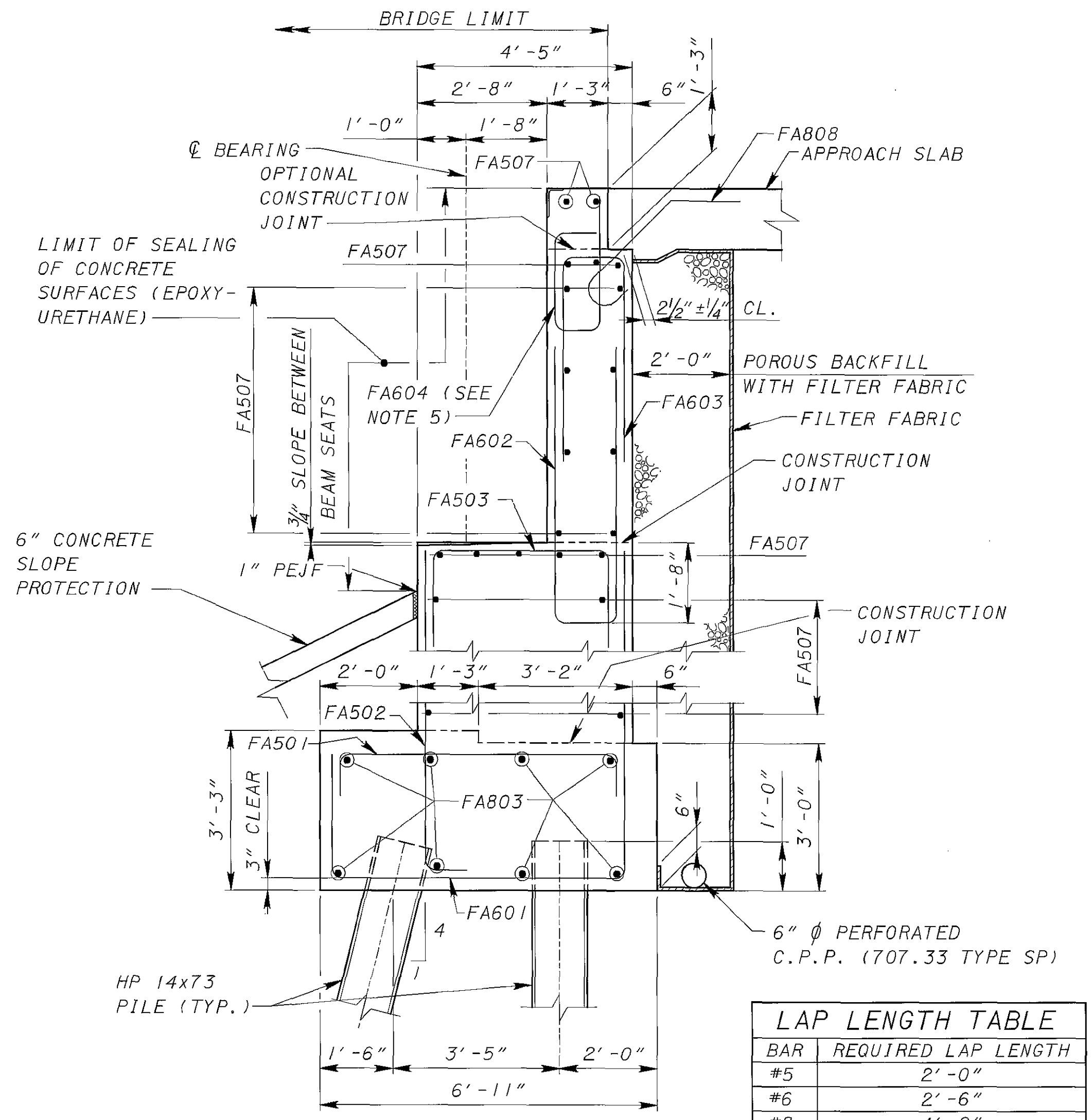
- LEGEND:**
- (BXX) - BEAM NUMBER
 - E.F. - EACH FACE
 - F.F. - FAR FACE
 - N.F. - NEAR FACE
 - P.E.J.F. - PREFORMED EXPANSION JOINT FILLER
 - C.P.P. - CORRUGATED POLYETHYLENE PIPE
 - U.N.O. - UNLESS NOTED OTHERWISE

- NOTES:**
1. FOR ADDITIONAL DETAIL NOTES REFER TO SHEET [15/55].
 2. FOR FOOTING PLAN AND SECTION A-A, SEE SHEET [20/55].
 3. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [20/55].
 4. FOR ELEVATION B-B AND C-C, SEE SHEET [21/55].
 5. FOR BEARING DEVICES, SEE SHEET [49/55].
 6. FOR APPROACH SLAB DETAILS, SEE SHEET [51/55].
 7. FOR REINFORCING STEEL LIST, SEE SHEET [53/55].
 8. FOR LAP LENGTH TABLE, SEE SHEET [17/55].
 9. FOR EXPANSION JOINT DETAILS, SEE SHEET [50/55].

Plotted By: ecmack Date: 7/27/2007 Time: 3:41:31 PM
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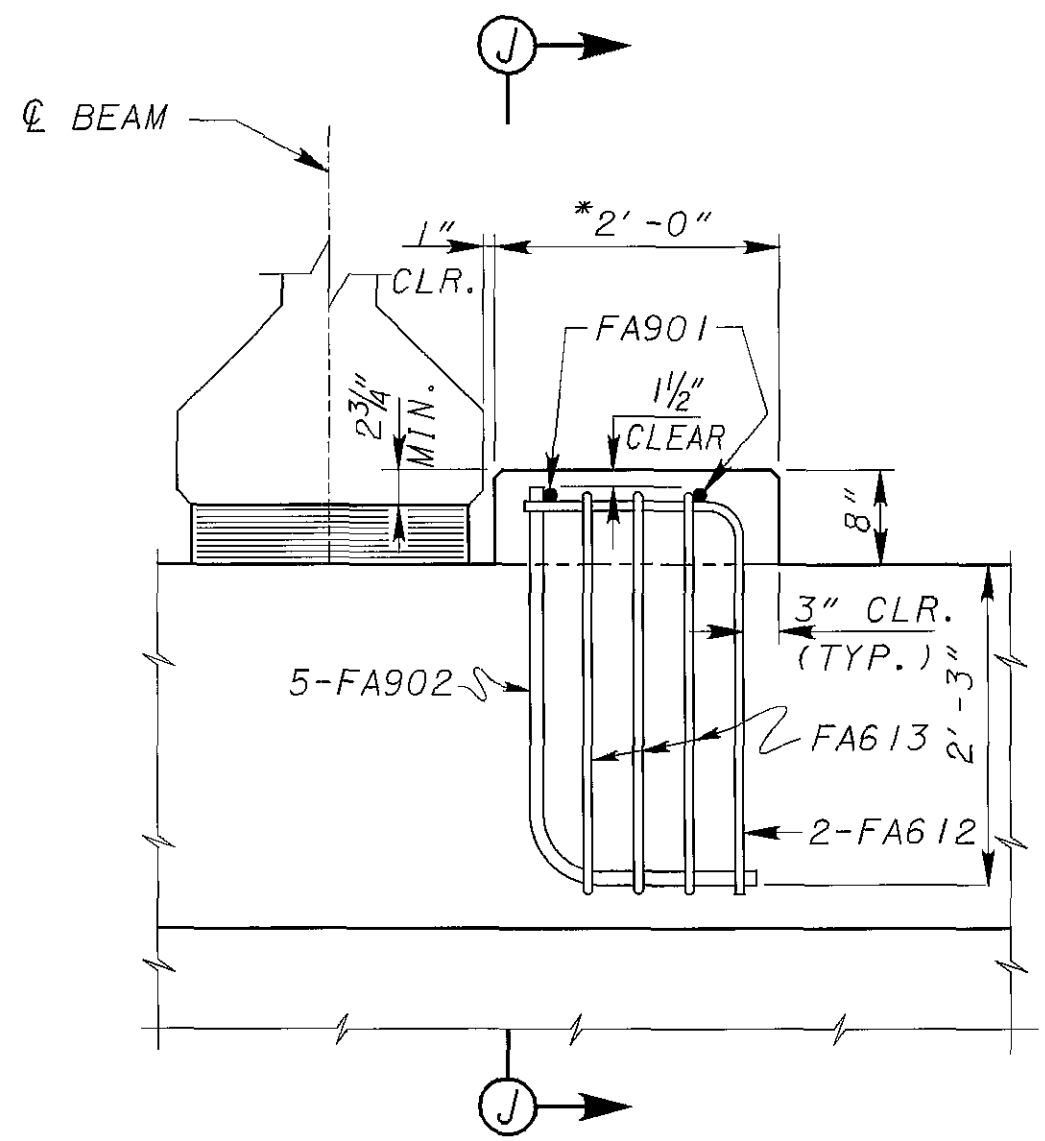


FOOTING PLAN



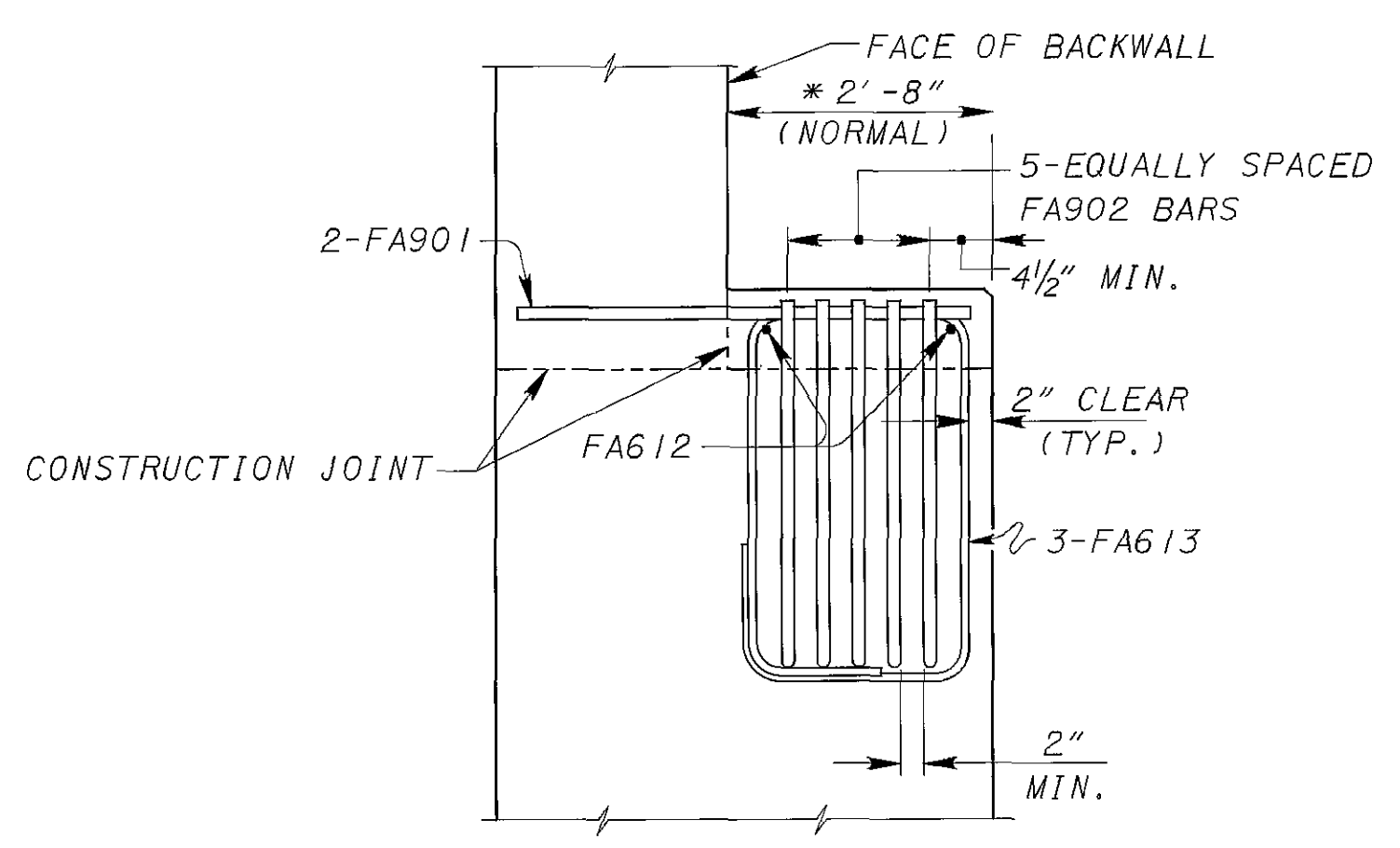
SECTION A-A
 (EXPANSION JOINT ANCHOR NOT SHOWN)

BAR	REQUIRED LAP LENGTH
#5	2'-0"
#6	2'-6"
#8	4'-0"



FRONT VIEW OF SEISMIC PEDESTAL

THE 2'-0" WIDTH OF THE PEDESTAL SHALL BE MEASURED PARALLEL TO THE CENTERLINE OF BEARING. THE FA902 AND FA612 BARS SHALL BE PLACED PARALLEL TO THE CENTERLINE OF BEARING. THE FA901 AND FA613 BARS SHALL BE PLACED PARALLEL TO THE BEAMS OR GIRDERS.



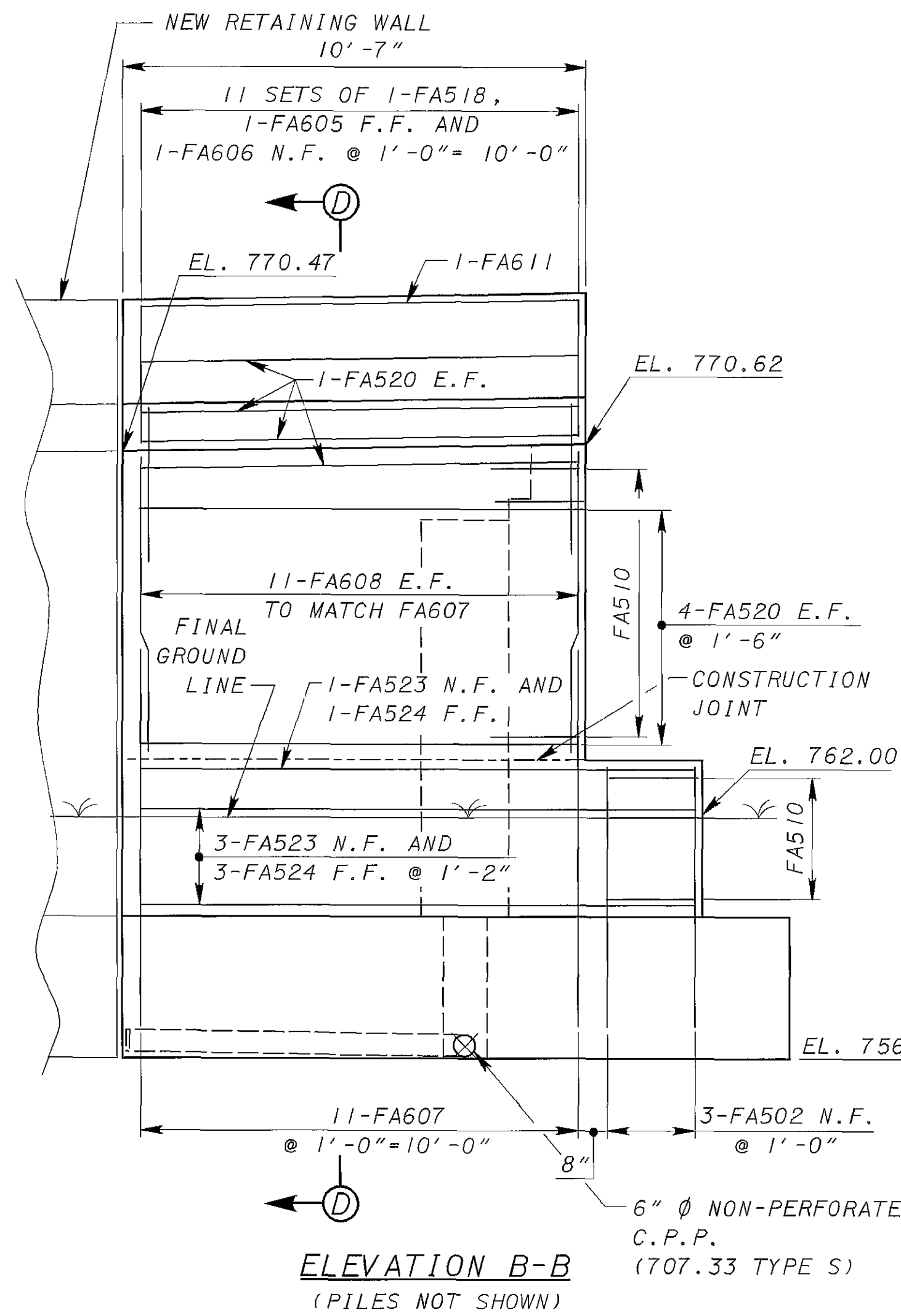
SECTION J-J

THE LOCATION OF THE MAIN REINFORCEMENT IN THE BEAM SEAT MAY BE ADJUSTED HORIZONTALLY ±1" TO ACCOMMODATE THE FA902 BARS.

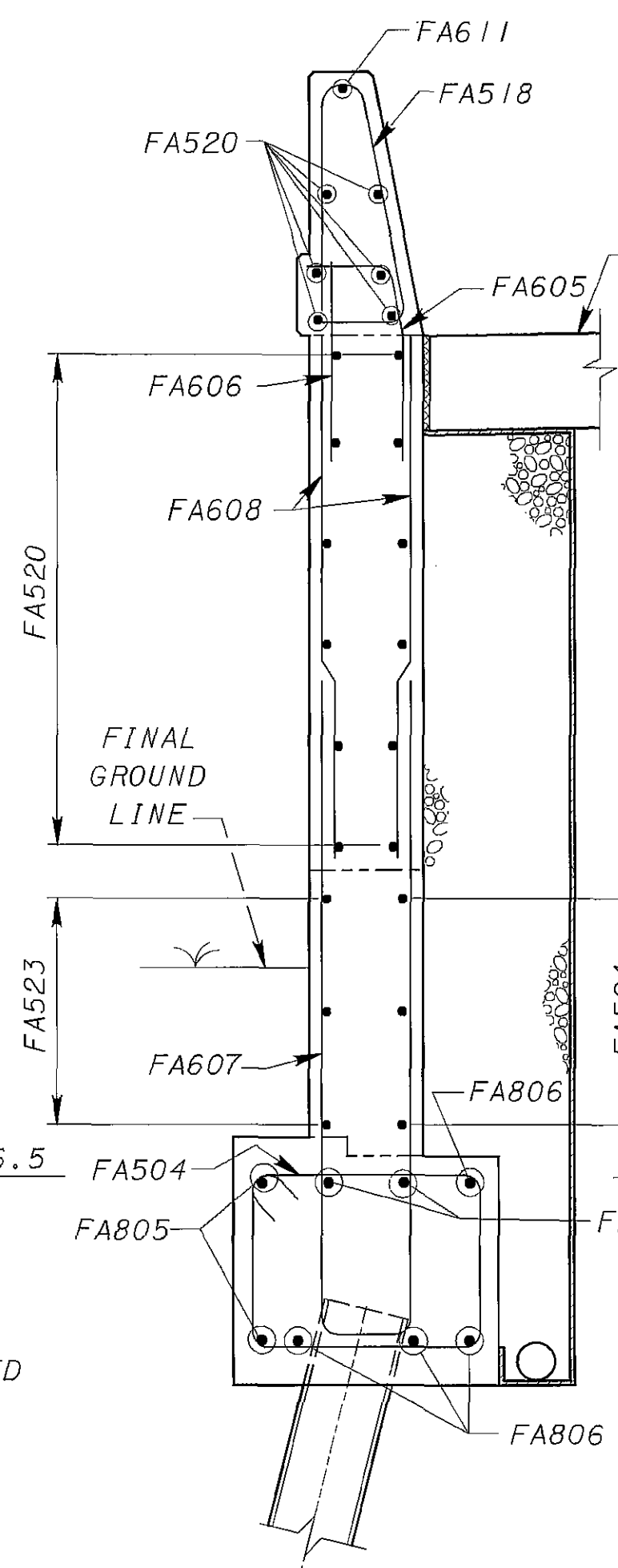
* - THE SURFACE OF THE BEAM SEAT IN THIS AREA SHALL BE FINISHED WITH A SERRATED TROWEL. THE SERRATIONS SHALL BE 1/4" DEEP MINIMUM.

NOTES:

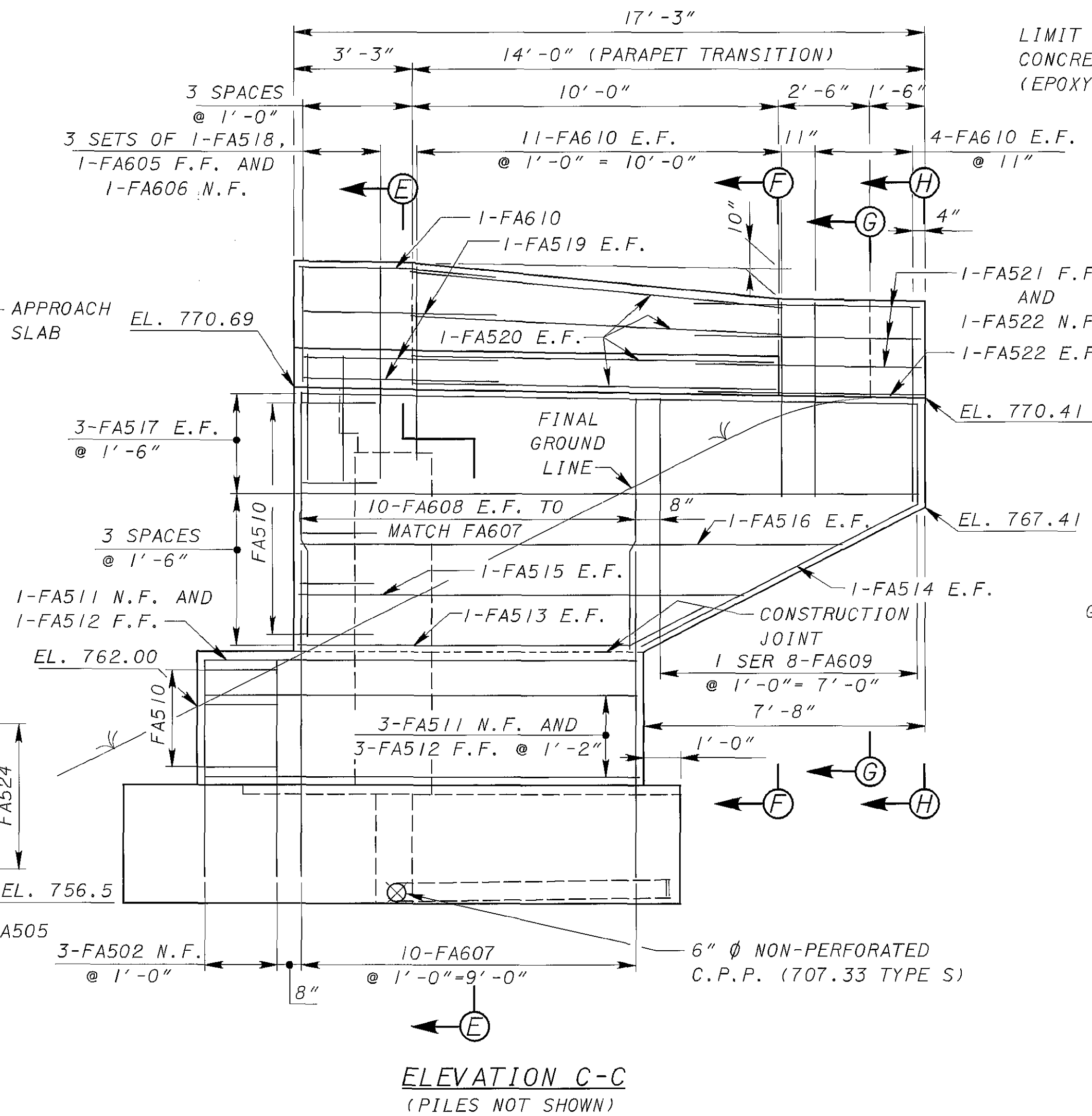
- FOR ABUTMENT PLAN AND ELEVATION, SEE SHEET [19]55.
- FOR ADDITIONAL NOTES AND LEGEND, SEE SHEET [19]55.
- FOR PILE LOCATIONS, SEE SHEETS [13]55 AND [14]55.
- FOR LOCATION OF SEISMIC PEDESTAL, SEE SHEET [19]55.
- FOR PLACEMENT OF FA604 BARS, SEE SHEET [50]55.
- FOR NON-PERFORATED PIPE TERMINATION IN SIDE SLOPE DETAIL, SEE STANDARD DRAWING A-1-69, SHEET [11]5.



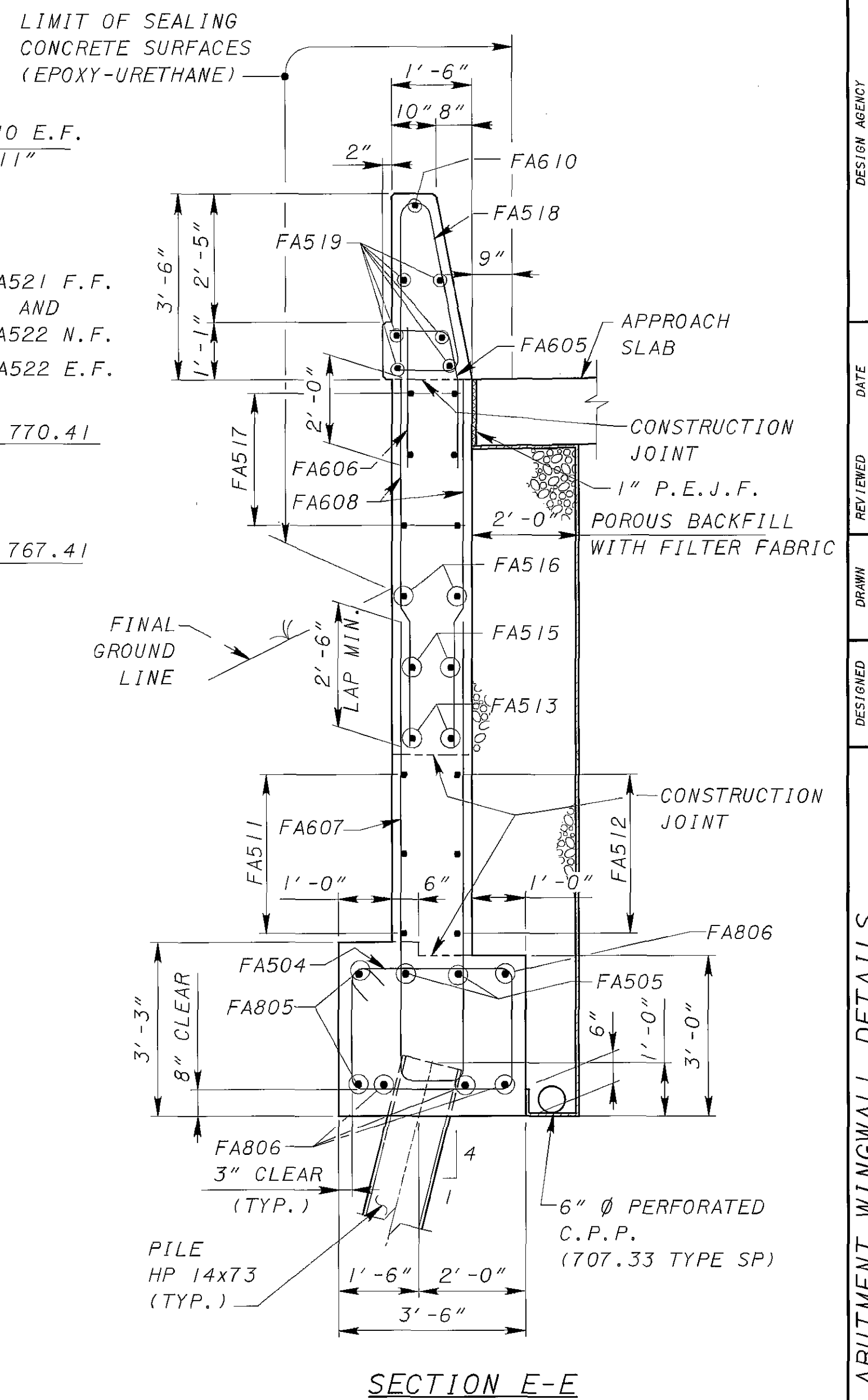
ELEVATION B-B
(PILES NOT SHOWN)



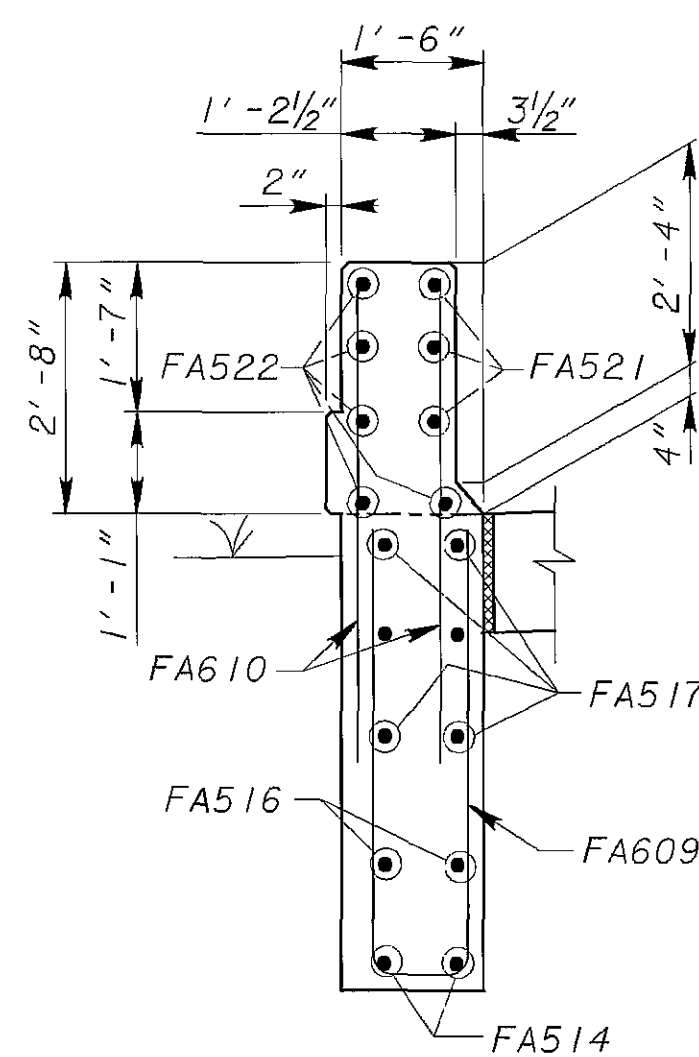
SECTION D-D
(FOR DETAILS NOT SHOWN,
SEE SECTION E-E)



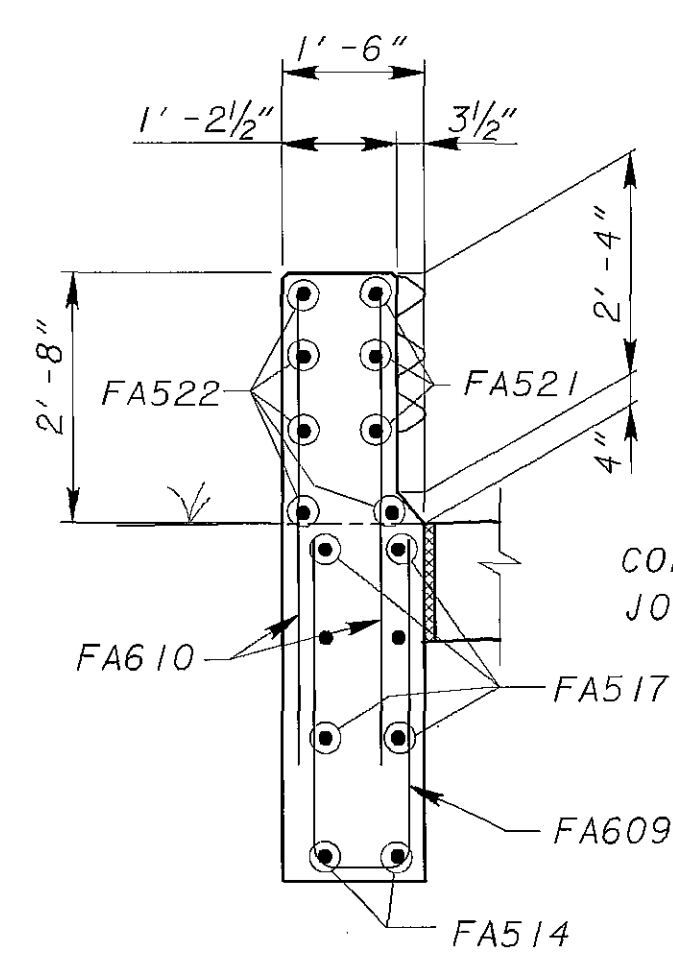
ELEVATION C-C
(PILES NOT SHOWN)



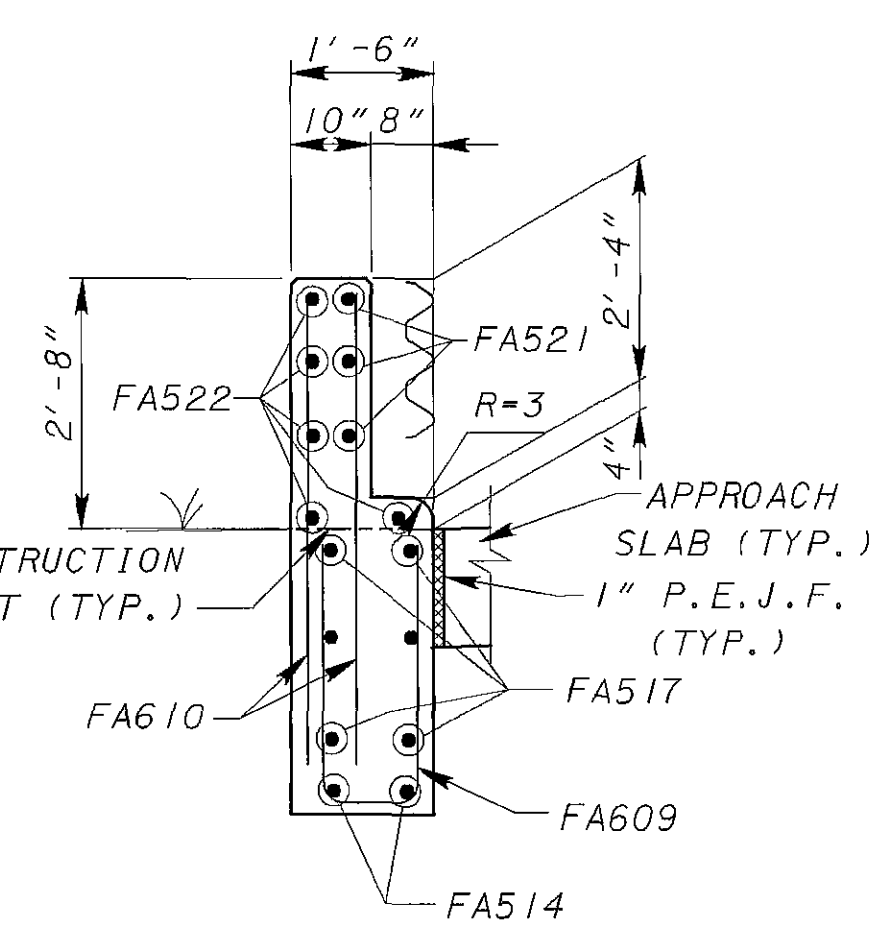
SECTION E-E



SECTION F-F



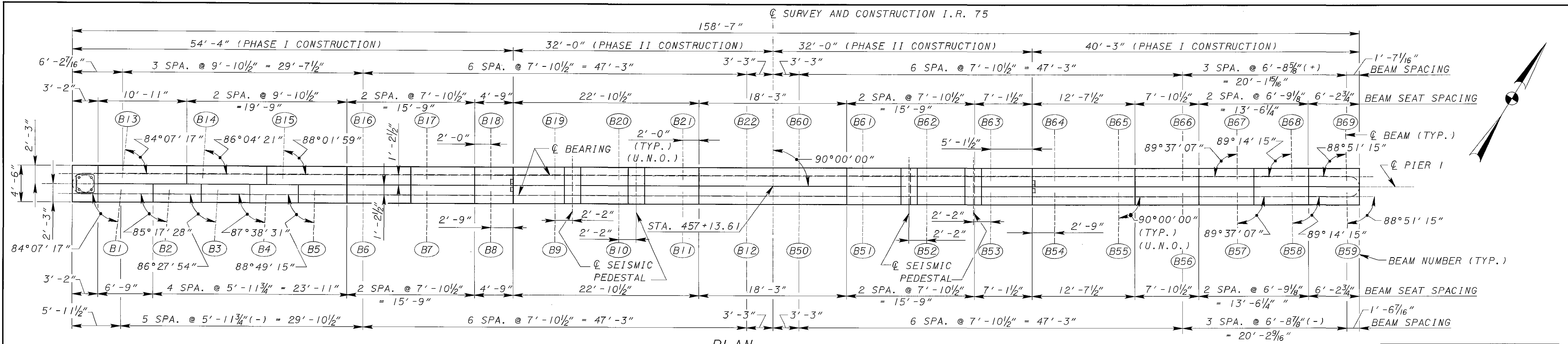
SECTION G-G



SECTION H-H

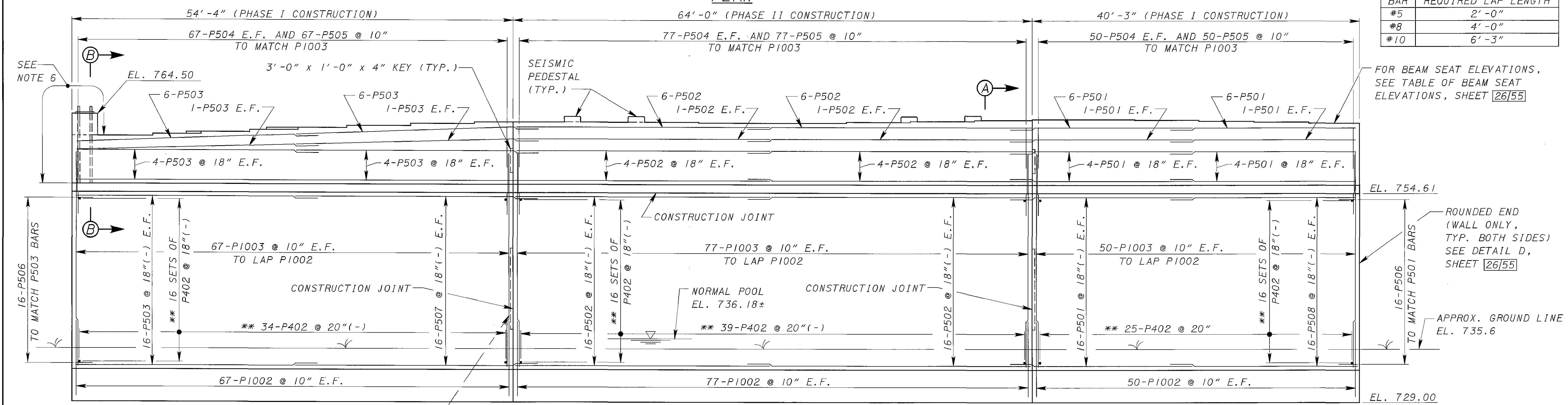
NOTES:

1. FOR ABUTMENT DETAILS, SEE SHEETS [19]55 AND [20]55.
2. FOR ADDITIONAL NOTES, SEE SHEET [15]55 AND [17]55.



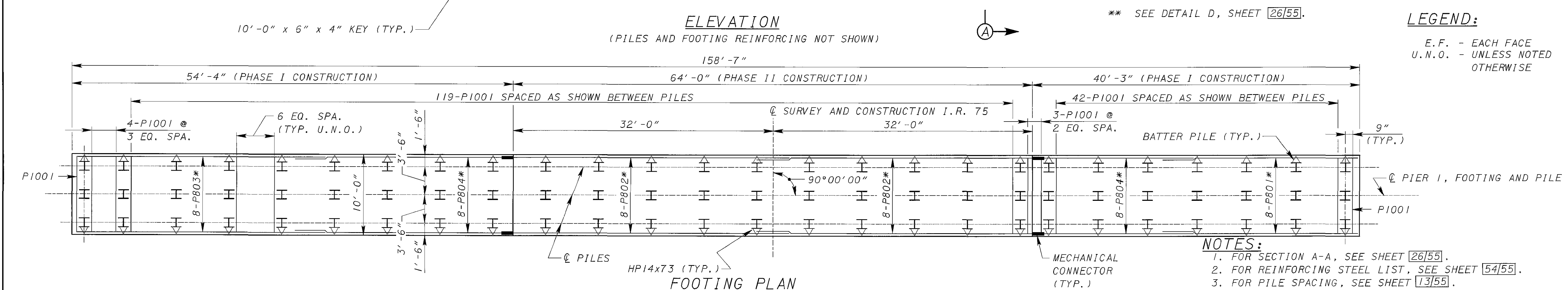
PLAN

BAR	REQUIRED LAP LENGTH
#5	2'-0"
#8	4'-0"
#10	6'-3"



ELEVATION

(PILES AND FOOTING REINFORCING NOT SHOWN)



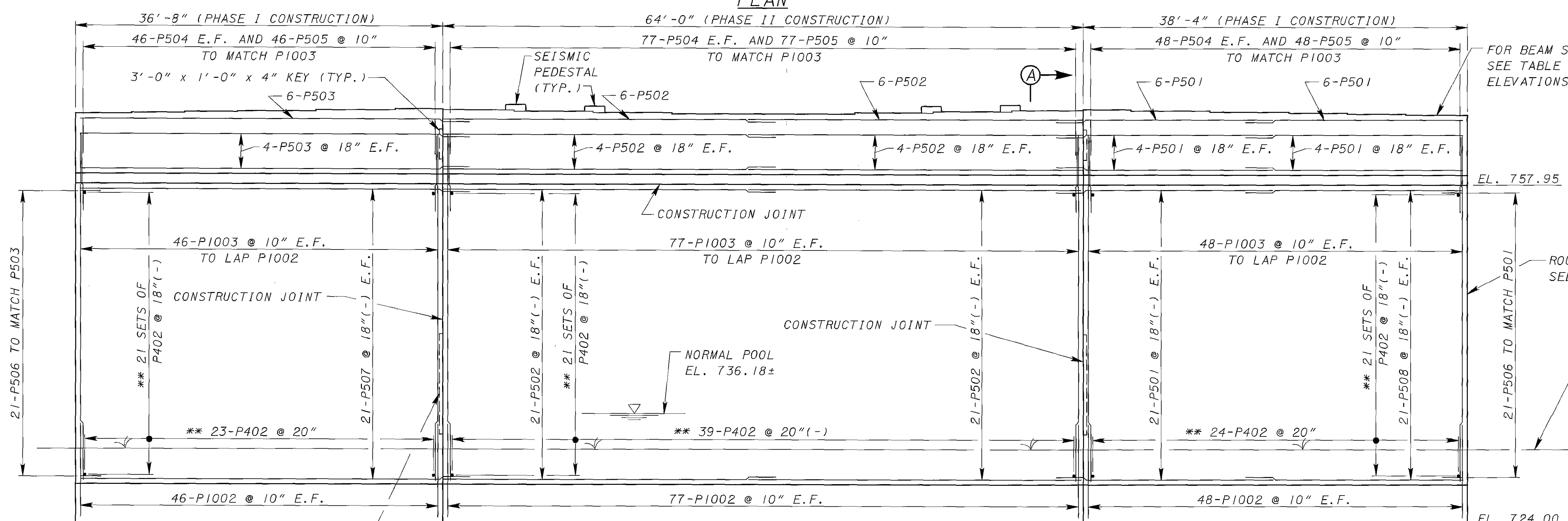
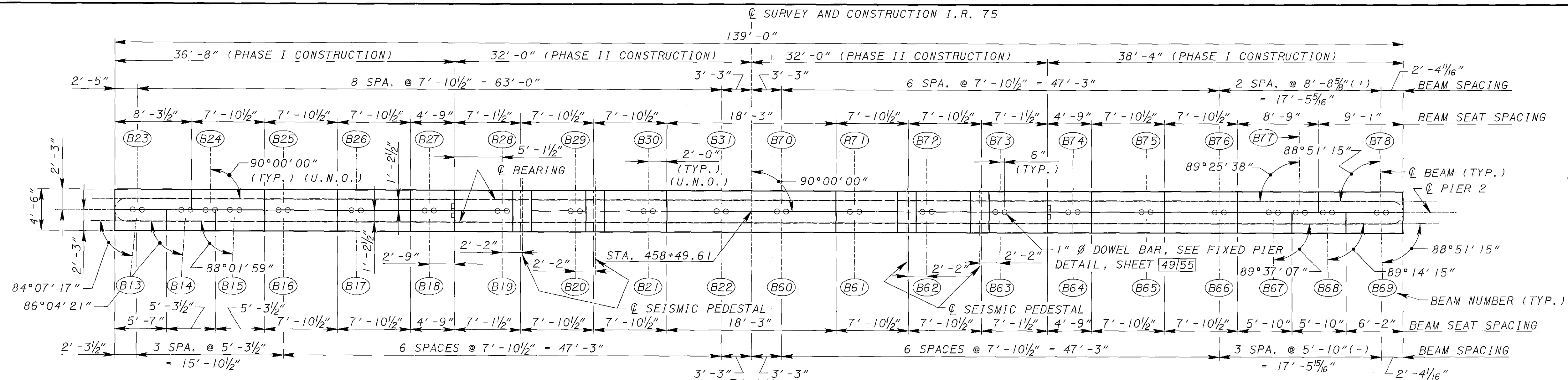
FOOTING PLAN

* - SPACED AS SHOWN IN SECTION A-A

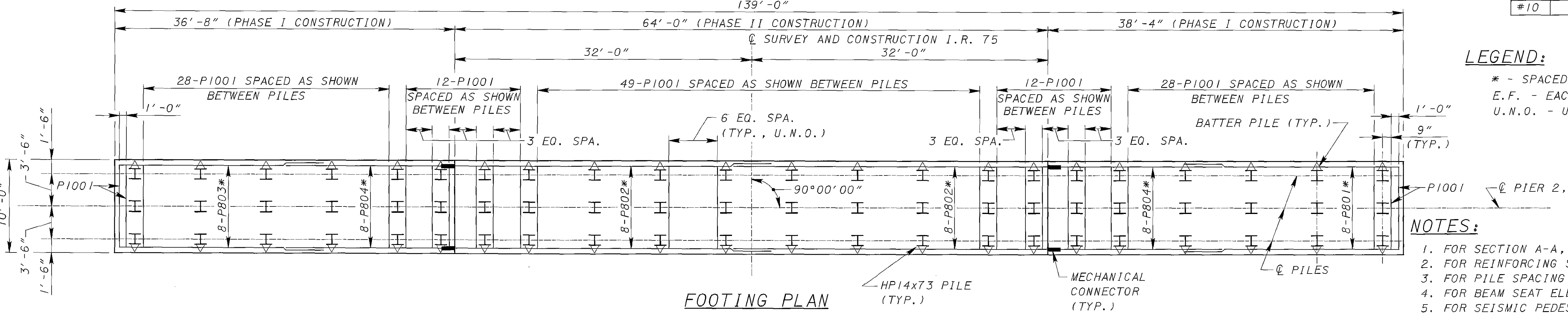
LEGEND:
 E.F. - EACH FACE
 U.N.O. - UNLESS NOTED OTHERWISE

- NOTES:**
1. FOR SECTION A-A, SEE SHEET [26/55].
 2. FOR REINFORCING STEEL LIST, SEE SHEET [54/55].
 3. FOR PILE SPACING, SEE SHEET [13/55].
 4. FOR BEAM SEAT ELEVATIONS, SEE SHEET [26/55].
 5. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [37/55].
 6. LIMITS OF SEALING CONCRETE SURFACES, (EPOXY-URETHANE) AROUND SIGN POST FOUNDATION, SEE GENERAL NOTES [3/55].

Plotted By: emack Date: 7/27/2007 Time: 3:42:06 PM
 Filename: g:\0004\0066\bridge\concrete.aif\cadd\23828p102.dgn



BAR	REQUIRED LAP LENGTH
#5	2'-0"
#8	4'-0"
#10	6'-3"



LEGEND:
 * - SPACED AS SHOWN IN SECTION A-A
 E.F. - EACH FACE
 U.N.O. - UNLESS NOTED OTHERWISE

- NOTES:
1. FOR SECTION A-A, SEE SHEET 26155.
 2. FOR REINFORCING STEEL LIST, SEE SHEET 54155.
 3. FOR PILE SPACING, SEE SHEET 13155.
 4. FOR BEAM SEAT ELEVATIONS, SEE SHEET 26155.
 5. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET 37155.

DESIGN AGENCY
TransSystems
 10000 W. 130th St., Suite 100
 Cleveland, Ohio 44130-3400

DATE: 5/26/05
 REVIEWED: RER
 STRUCTURE FILE NUMBER: 5708710

DRAWN: CAG
 CHECKED: NFF

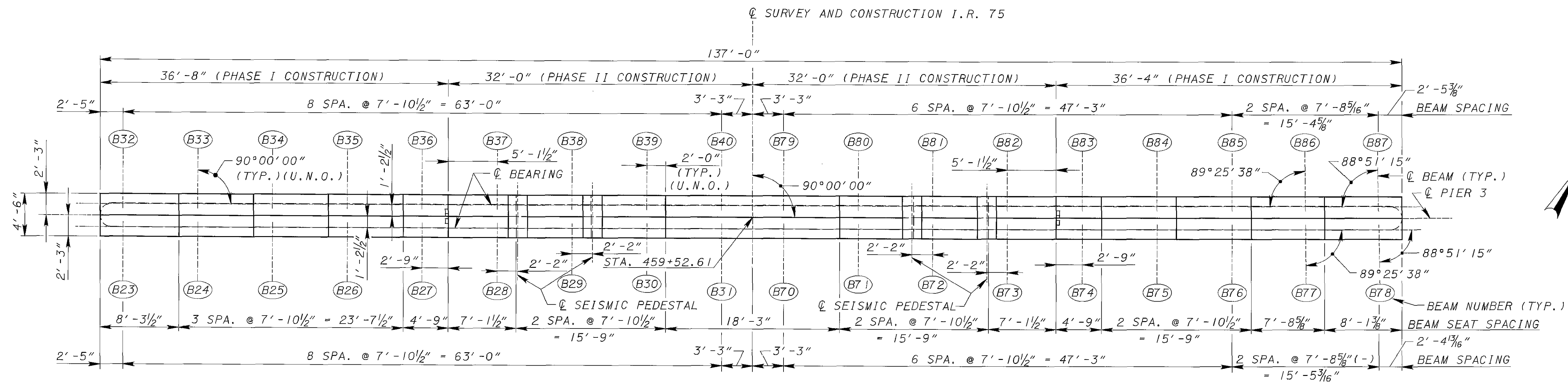
DESIGNED: JDH
 CHECKED: NFF

BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 IR-75 OVER THE GREAT MIAMI RIVER

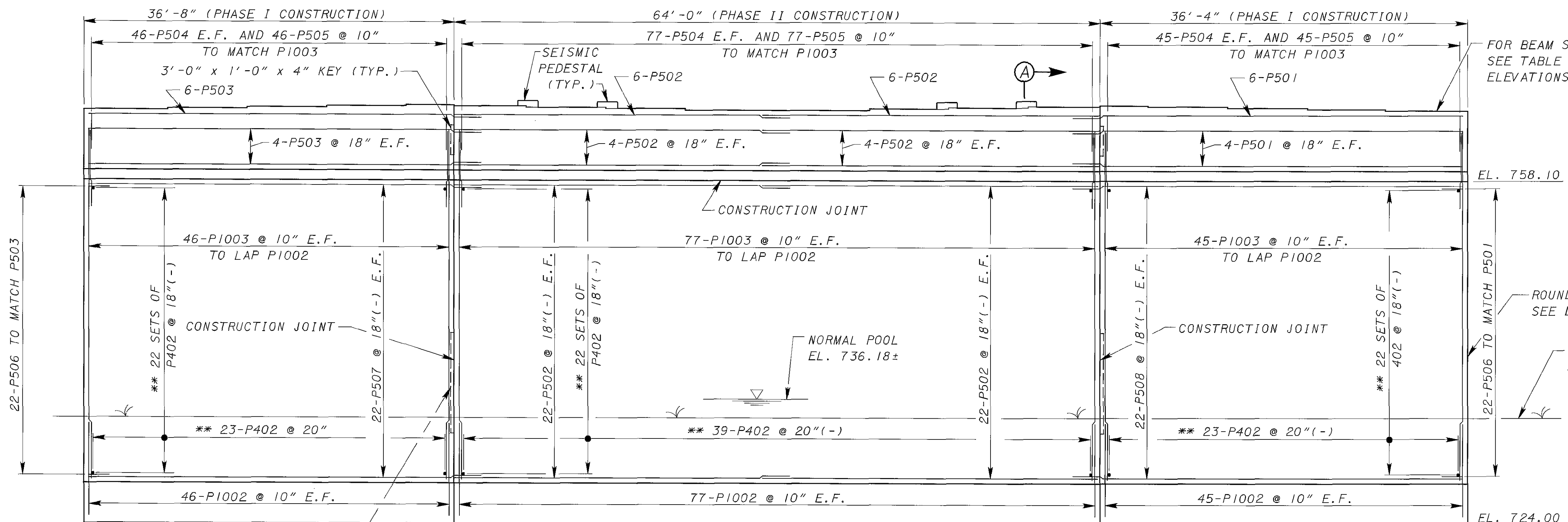
MOT-75-14.60
 PID 23828

23/55
 258/314

Plotted By: emack Date: 7/27/2007 Time: 3:42:07 PM
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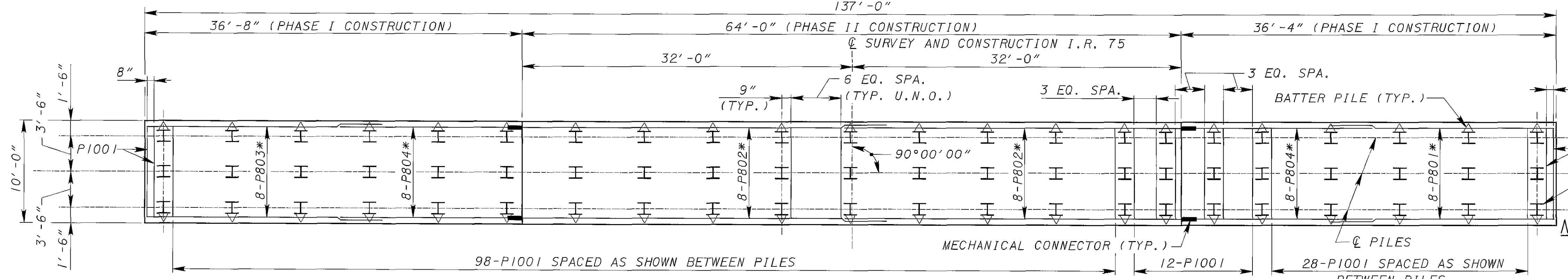
PLAN



ELEVATION

(PILES AND FOOTING REINFORCING NOT SHOWN)
 137'-0"

BAR	REQUIRED LAP LENGTH
#5	2'-0"
#8	4'-0"
#10	6'-3"



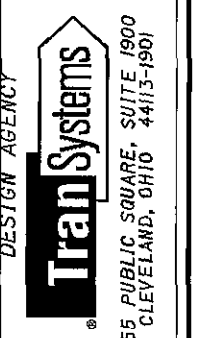
FOOTING PLAN

LEGEND:

- * - SPACED AS SHOWN IN SECTION A-A
- E.F. - EACH FACE
- U.N.O. - UNLESS NOTED OTHERWISE

NOTES:

1. FOR SECTION A-A, SEE SHEET [26/55].
2. FOR REINFORCING STEEL LIST, SEE SHEET [54/55].
3. FOR PILE SPACING, SEE SHEET [14/55].
4. FOR BEAM SEAT ELEVATIONS, SEE SHEET [26/55].
5. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [37/55].



DESIGN FIRM
TranSystems
 65 PUBLIC SQUARE, SUITE 1900
 CLEVELAND, OHIO 44115-9601

REVIEWED DATE 5/26/05
 RER STRUCTURE FILE NUMBER 5708710

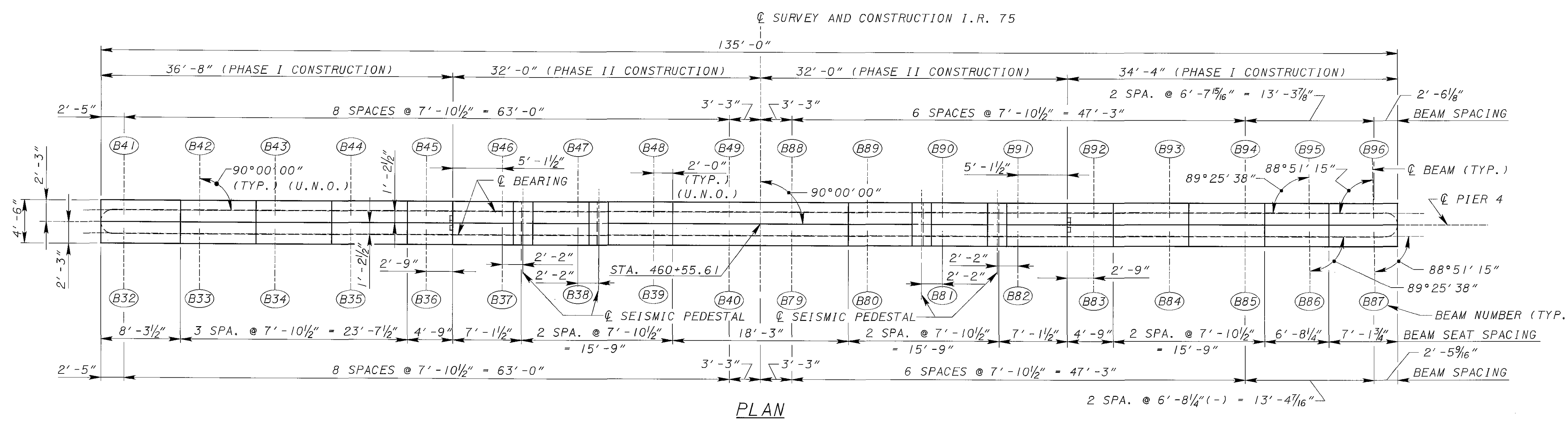
DRAWN CAG
 CHECKED NFF

DESIGNED JDH
 CHECKED NFF

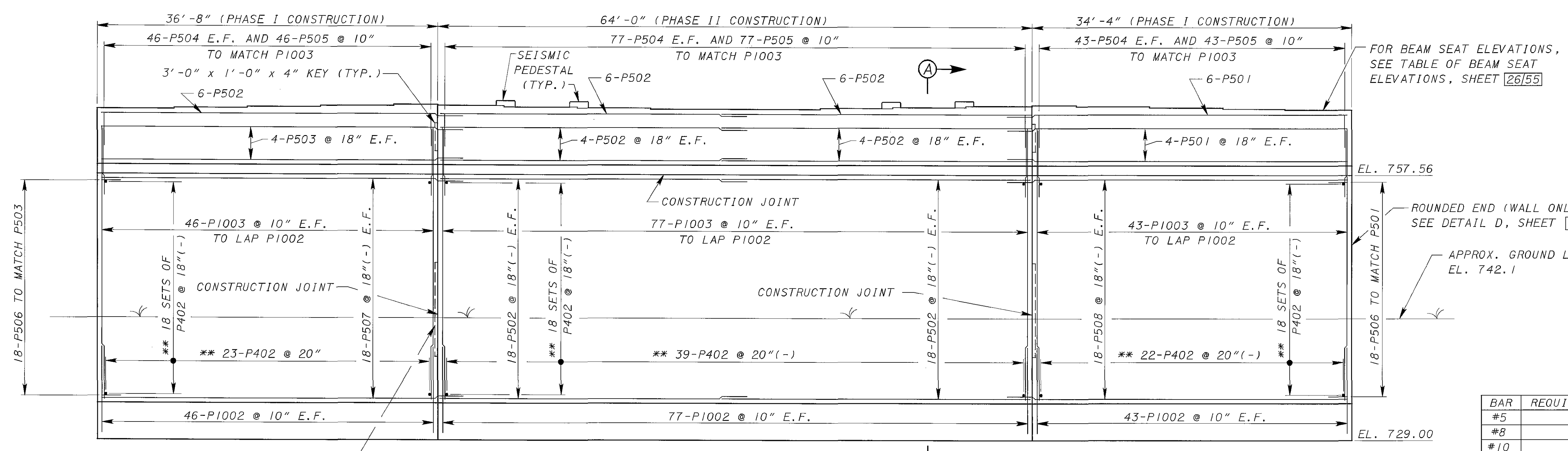
BRIDGE NO. MOT-75-1523
 IR-75 OVER THE GREAT MIAMI RIVER

PIER 3 DETAILS
 (PRESTRESSED CONCRETE OPTION)

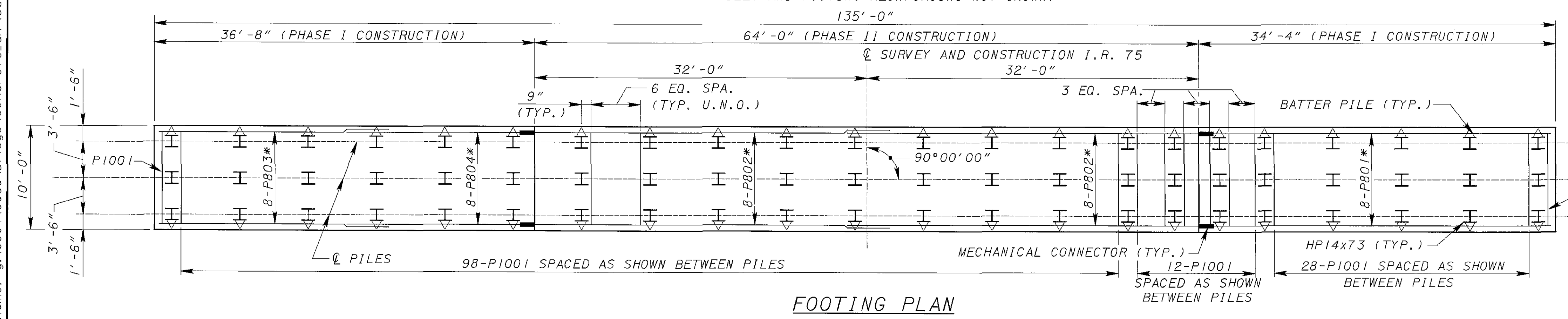
MOT-75-14.60
 PID 23828
 24/55
 259
 314



PLAN



ELEVATION
 (PILES AND FOOTING REINFORCING NOT SHOWN)



FOOTING PLAN
 * - SPACED AS SHOWN IN SECTION A-A

LEGEND:

- E.F. - EACH FACE
- U.N.O. - UNLESS NOTED OTHERWISE

NOTES:

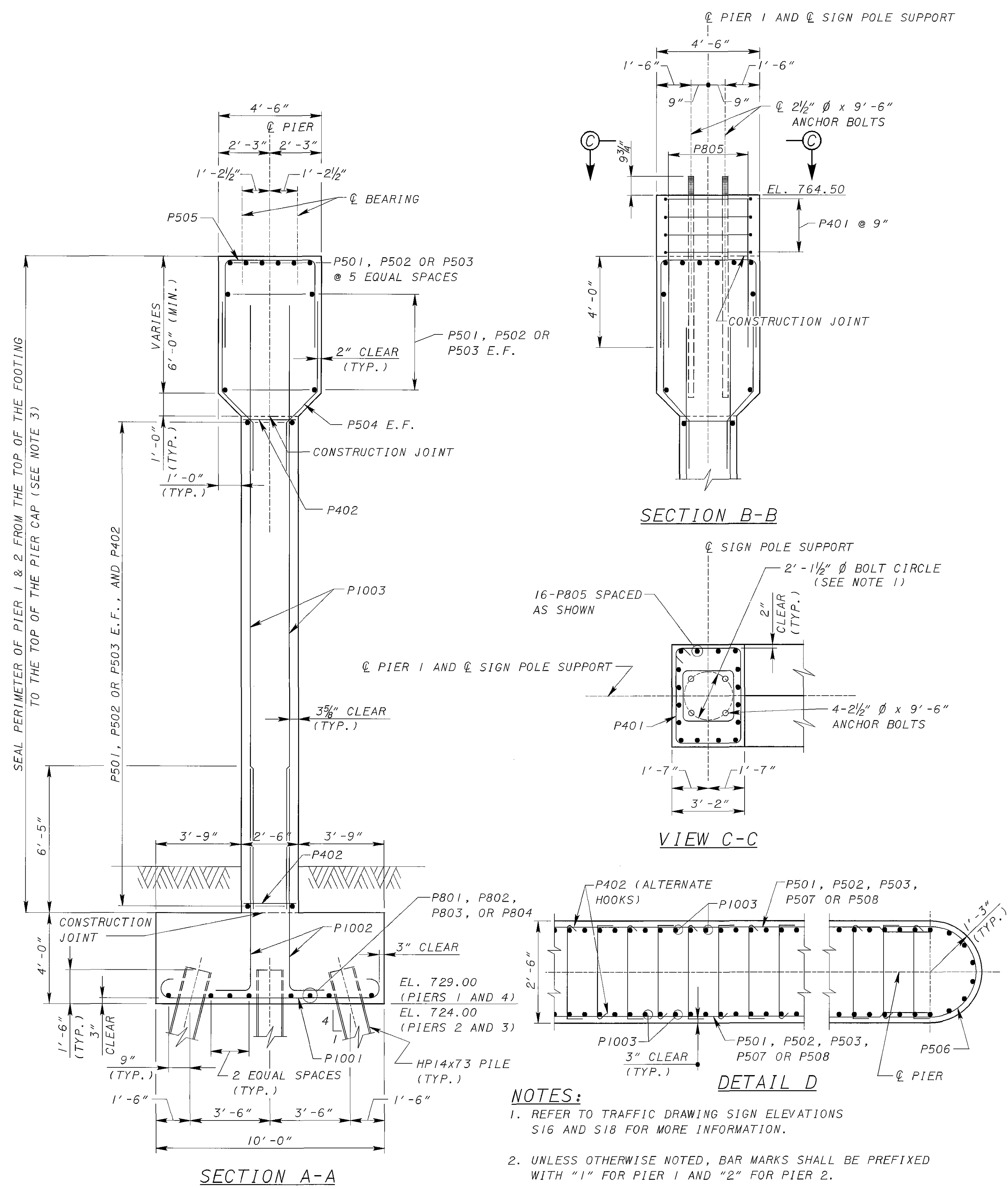
1. FOR SECTION A-A, SEE SHEET [26/55].
2. FOR REINFORCING STEEL LIST, SEE SHEET [54/55].
3. FOR PILE SPACING, SEE SHEET [14/55].
4. FOR BEAM SEAT ELEVATIONS, SEE SHEET [26/55].
5. FOR SEISMIC PEDESTAL DETAILS, SEE SHEET [37/55].

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Plotted By: ecmack Date: 7/27/2007 Time: 3:42:08 PM
 Filename: g:\c004\0066\bridge\concrete.alt\cadd\23828p105.dgn

TABLE OF BEAM SEAT ELEVATIONS

LEFT BRIDGE					RIGHT BRIDGE				
SPAN 1					SPAN 1				
BEAM NUMBER	BEARING LOCATION	BEAM SEAT ELEVATION	BEARING LOCATION	BEAM SEAT ELEVATION	BEAM NUMBER	BEARING LOCATION	BEAM SEAT ELEVATION	BEARING LOCATION	BEAM SEAT ELEVATION
B1	REAR ABUTMENT	757.66	PIER 1	761.61	B50	REAR ABUTMENT	761.00	PIER 1	763.50
B2		758.23		761.91	B51		761.13		763.63
B3		758.79		762.21	B52		761.25		763.75
B4		759.36		762.51	B53		761.38		763.88
B5		759.92		762.81	B54		761.51		763.99
B6		760.24		763.11	B55		761.64		763.99
B7		760.48		763.42	B56		761.62		763.91
B8		760.70		763.61	B57		761.50		763.80
B9		760.84		763.62	B58		761.37		763.70
B10		760.97		763.62	B59		761.25		763.59
B11		761.09		763.61					
B12		761.00		763.50					
SPAN 2					SPAN 2				
B13	PIER 1	761.68	PIER 2	764.98	B60	PIER 1	763.53	PIER 2	765.00
B14		762.18		765.06	B61		763.66		765.13
B15		762.67		765.15	B62		763.78		765.25
B16		763.16		765.23	B63		763.91		765.38
B17		763.46		765.36	B64		764.02		765.48
B18		763.65		765.48	B65		764.01		765.36
B19		763.66		765.38	B66		763.94		765.23
B20		763.65		765.25	B67		763.83		765.14
B21		763.64		765.13	B68		763.72		765.04
B22		763.53		765.00	B69		763.61		764.95
SPAN 3					SPAN 3				
B23	PIER 2	765.08	PIER 3	765.11	B70	PIER 2	765.10	PIER 3	765.13
B24		765.20		765.23	B71		765.23		765.26
B25		765.33		765.36	B72		765.35		765.38
B26		765.46		765.49	B73		765.48		765.51
B27		765.58		765.61	B74		765.58		765.61
B28		765.48		765.51	B75		765.46		765.49
B29		765.35		765.38	B76		765.33		765.36
B30		765.23		765.26	B77		765.19		765.24
B31		765.10		765.13	B78		765.05		765.11
SPAN 4					SPAN 4				
B32	PIER 3	765.10	PIER 4	764.58	B79	PIER 3	765.12	PIER 4	764.60
B33		765.22		764.70	B80		765.25		764.73
B34		765.35		764.83	B81		765.37		764.85
B35		765.48		764.96	B82		765.50		764.98
B36		765.60		765.08	B83		765.60		765.08
B37		765.50		764.98	B84		765.48		764.96
B38		765.37		764.85	B85		765.35		764.83
B39		765.25		764.73	B86		765.23		764.72
B40		765.12		764.60	B87		765.10		764.62
SPAN 5					SPAN 5				
B41	PIER 4	764.56	FORWARD ABUTMENT	763.37	B88	PIER 4	764.58	FORWARD ABUTMENT	763.39
B42		764.68		763.49	B89		764.71		763.52
B43		764.81		763.62	B90		764.83		763.64
B44		764.94		763.75	B91		764.96		763.77
B45		765.06		763.87	B92		765.06		763.87
B46		764.96		763.77	B93		764.94		763.75
B47		764.83		763.64	B94		764.81		763.62
B48		764.71		763.52	B95		764.70		763.53
B49		764.58		763.39	B96		764.60		763.44



- NOTES:**
- REFER TO TRAFFIC DRAWING SIGN ELEVATIONS S16 AND S18 FOR MORE INFORMATION.
 - UNLESS OTHERWISE NOTED, BAR MARKS SHALL BE PREFIXED WITH "1" FOR PIER 1 AND "2" FOR PIER 2.
 - FOR SEALING OF CONCRETE SURFACES (EPOXY URETHANE) REQUIREMENTS, SEE GENERAL NOTES SHEET 3/55.
 - FOR PILE LAYOUT, SEE PILE LAYOUT PLAN, SHEETS 13/55 AND 14/55.

DESIGN AGENCY
Tran Systems
 5000 W. 11th Ave., Suite 100
 Denver, CO 80202

DATE	5/26/05
REVIEWED	RER
STRUCTURE FILE NUMBER	5708710
DRAWN	JDH
CHECKED	NFF
DESIGNED	NFF

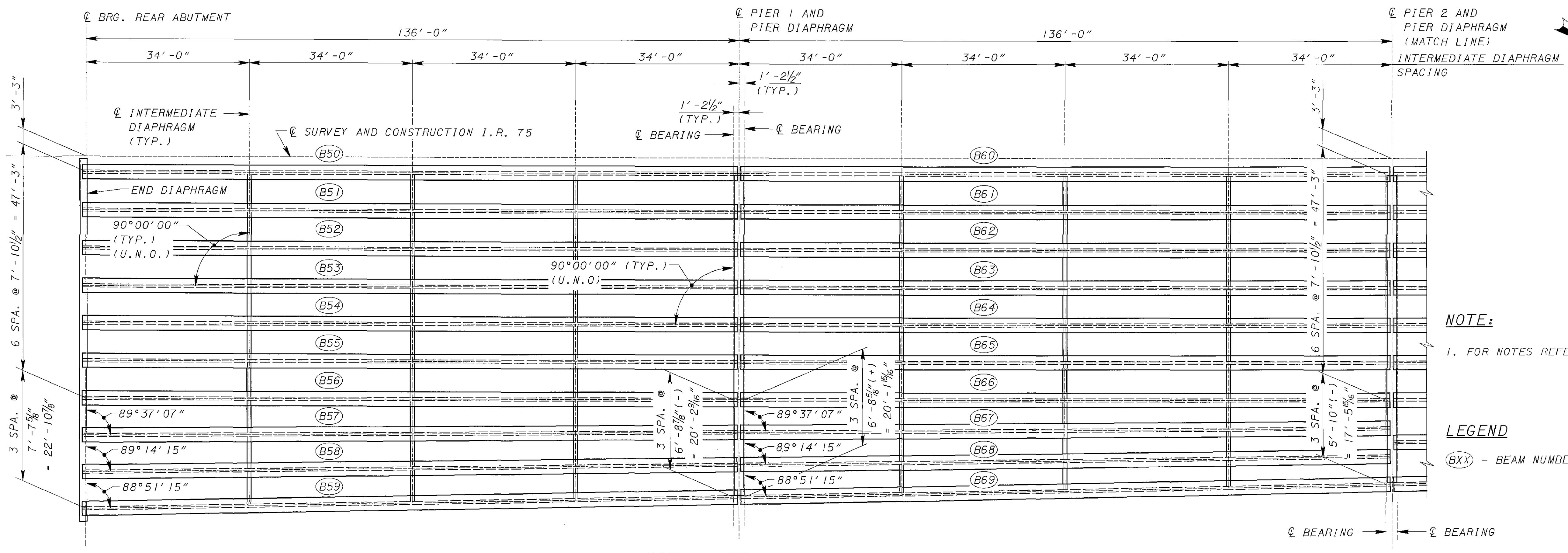
PIER SECTIONS AND DETAILS (PRESTRESSED CONCRETE OPTION)

BRIDGE NO. MOT-75-1523
 IR-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

26/55
 261
 314

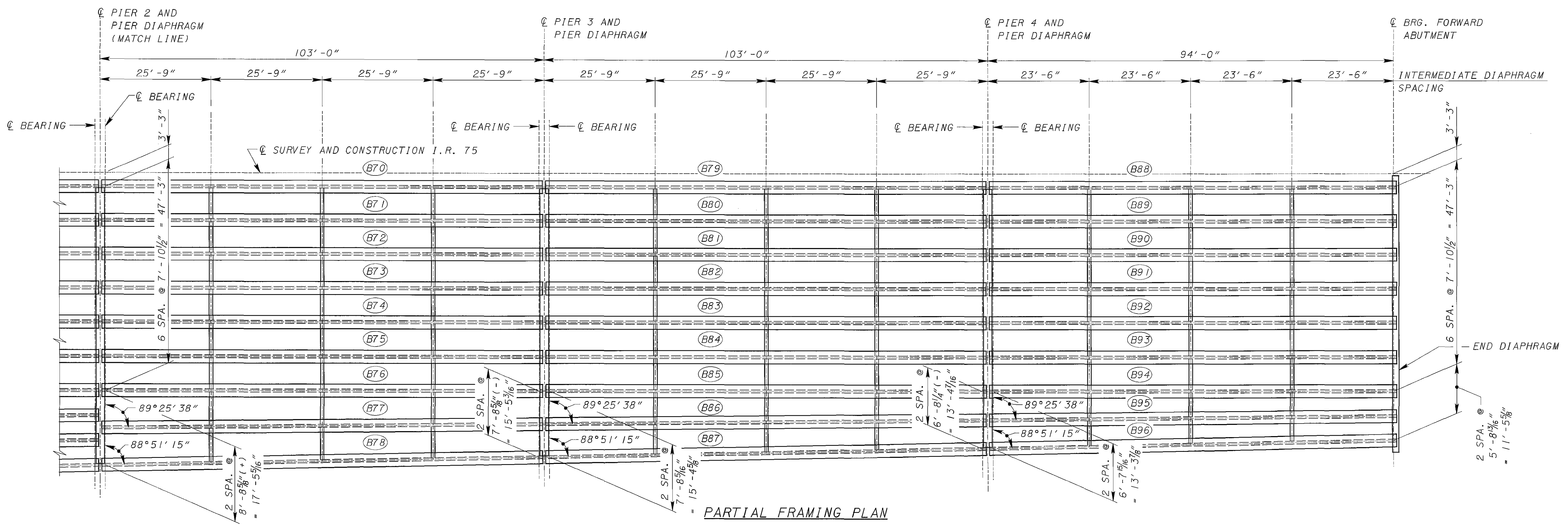
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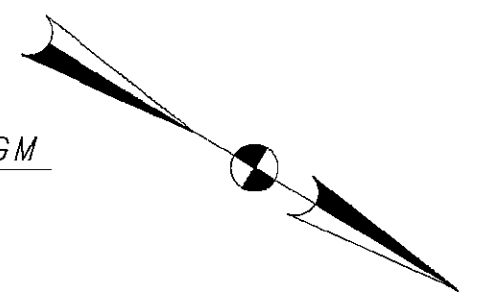
PARTIAL FRAMING PLAN

NOTE:
 1. FOR NOTES REFER TO SHEET [27]55.

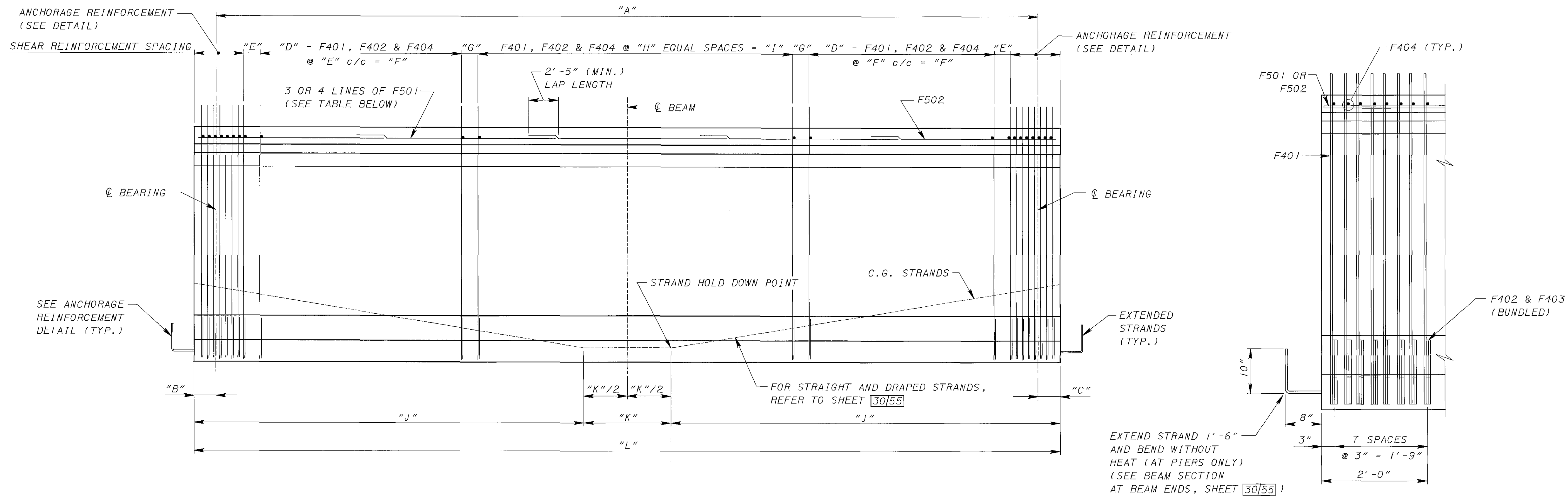
LEGEND
 (BXX) = BEAM NUMBER



PARTIAL FRAMING PLAN



DESIGN AGENCY TranSystems CLEVELAND, OHIO 44115-9900	
DESIGNED J.D.H.	DATE 05/27/05
DRAWN C.A.G.	REVIEWED RER
CHECKED M.F.F.	STRUCTURE FILE NUMBER 5708710
FRAMING PLAN - RIGHT BRIDGE BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION) I.R. 75 OVER THE GREAT MIAMI RIVER	
MOT-75-14.60 PID 23828	
28/55	
263 314	



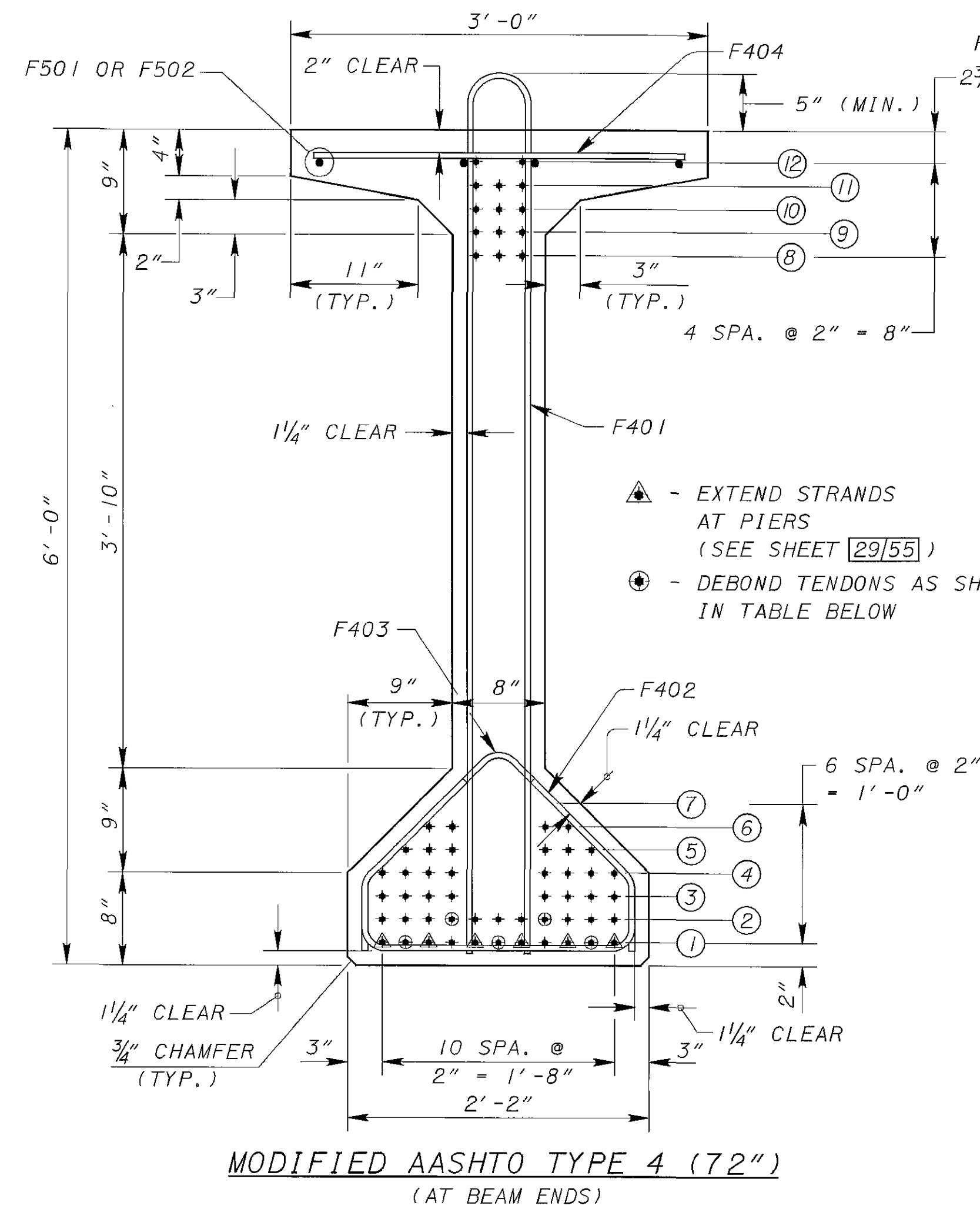
TYPICAL BEAM ELEVATION
 (THREADED INSERTS AND 1/4" Ø SLEEVES FOR DIAPHRAGMS NOT SHOWN FOR CLARITY,
 REFER TO DIAPHRAGM DETAIL SHEETS [32/55] THRU [37/55] FOR LOCATIONS AND DETAILS)

ANCHORAGE REINFORCEMENT DETAIL

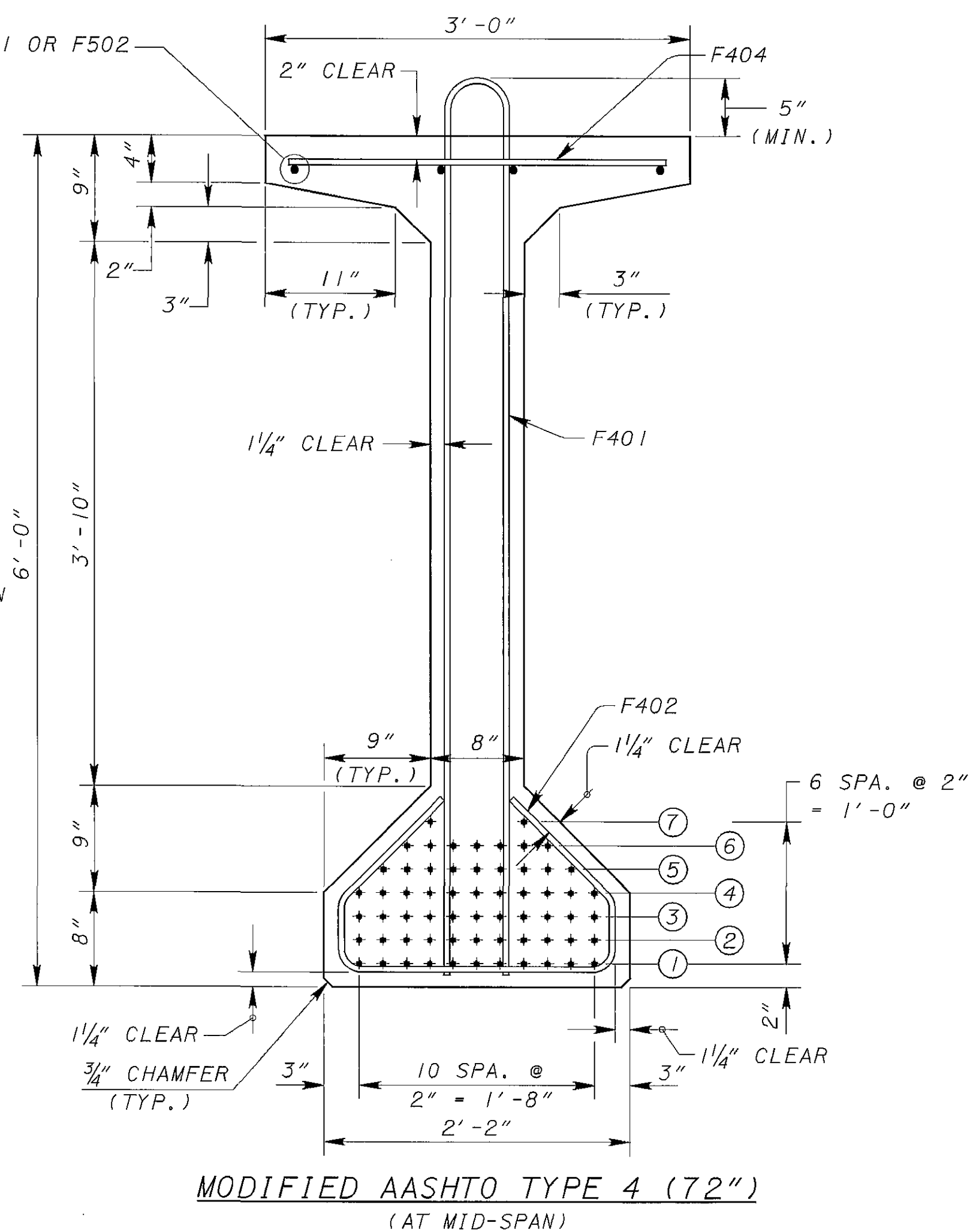
TABLE OF BEAM DIMENSIONS AND REINFORCEMENT

	BEAM NUMBER	NUMBER REQUIRED	DIMENSIONS												APPROXIMATE WEIGHT (LBS.)	F401 BARS REQ'D	F402 BARS REQ'D	F403 BARS REQ'D	F404 BARS REQ'D	NO. F501 LINES REQ'D	NO. F502 LINES REQ'D	F502 BARS LENGTH
			"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H"	"I"	"J"	"K"	"L"								
LEFT BRIDGE	(B1)	1	135'-6 1/16"	8 3/4"	10"	21	6"	10'-0"	1'-6"	75	112'-0 7/8"	61'-8 3/16"	13'-8 1/2"	137'-0 7/8"	136,525	134	134	16	134	4	1	26'-6"
	(B2)	1	135'-3"	8 3/4"	10"	21	6"	10'-0"	1'-6"	75	111'-9 3/4"	61'-7"	13'-8 3/16"	136'-9 3/4"	136,265	134	134	16	134	4	1	26'-6"
	(B3)	1	135'-0 9/16"	8 3/4"	10"	21	6"	10'-0"	1'-6"	74	111'-7 5/16"	61'-5 1/16"	13'-7 5/16"	136'-7 5/16"	136,063	133	133	16	133	4	1	26'-6"
	(B4)	1	134'-10 7/16"	8 3/4"	10"	21	6"	10'-0"	1'-6"	74	111'-5 5/8"	61'-5"	13'-7 3/4"	136'-5 5/8"	135,923	133	133	16	133	4	1	26'-6"
	(B5)	1	134'-9 9/16"	8 3/4"	10"	21	6"	10'-0"	1'-6"	74	111'-4 9/16"	61'-4 7/16"	13'-7 5/8"	136'-4 9/16"	135,835	133	133	16	133	4	1	26'-6"
	(B6) THRU (B12)	7	134'-9 1/2"	8 3/4"	10"	21	6"	10'-0"	1'-6"	74	111'-4 3/16"	61'-4 5/16"	13'-7 5/8"	136'-4 1/4"	135,809	133	133	16	133	4	1	26'-6"
	(B13)	1	134'-3 1/2"	10"	10"	41	1'-0"	40'-0"	1'-6"	33	49'-11 1/2"	61'-2 1/4"	13'-7 1/8"	135'-11 1/2"	135,414	132	132	16	132	4	1	26'-6"
	(B14)	1	133'-10 3/4"	10"	10"	41	1'-0"	40'-0"	1'-6"	33	49'-6 3/4"	61'-0"	13'-6 3/4"	135'-6 3/4"	135,020	132	132	16	132	4	1	26'-6"
	(B15)	1	133'-7 5/16"	10"	10"	41	1'-0"	40'-0"	1'-6"	33	49'-4"	60'-10 3/4"	13'-6 3/8"	135'-4"	134,792	132	132	16	132	4	1	26'-6"
	(B16) THRU (B22)	7	133'-7"	10"	10"	21	6"	10'-0"	1'-6"	74	110'-3"	60'-10 3/8"	13'-6 5/16"	135'-3"	134,709	133	133	16	133	4	1	26'-6"
(B23) THRU (B40)	18	100'-7"	10"	10"	7	1'-0"	6'-0"	1'-6"	56	84'-3"	46'-0 1/8"	10'-2 1/16"	102'-3"	101,841	87	87	16	87	3	1	19'-2"	
(B41) THRU (B49)	9	92'-9 1/2"	10"	9"	7	1'-0"	6'-0"	1'-6"	51	76'-4 1/2"	42'-5 5/8"	9'-5 1/4"	94'-4 1/2"	93,998	82	82	16	82	3	1	11'-4"	
RIGHT BRIDGE	(B50) THRU (B59)	10	134'-9 1/2"	8 3/4"	10"	21	6"	10'-0"	1'-6"	74	111'-4 3/16"	61'-4 5/16"	13'-7 5/8"	136'-4 1/4"	135,809	133	133	16	133	4	1	25'-9"
	(B60) THRU (B69)	10	133'-7"	10"	10"	21	6"	10'-0"	1'-6"	74	110'-3"	60'-10 3/8"	13'-6 5/16"	135'-3"	134,709	133	133	16	133	4	1	25'-9"
	(B70) THRU (B87)	18	100'-7"	10"	10"	7	1'-0"	6'-0"	1'-6"	56	84'-3"	46'-0 1/8"	10'-2 1/16"	102'-3"	101,841	87	87	16	87	3	1	19'-2"
	(B88) THRU (B96)	9	92'-9 1/2"	10"	9"	7	1'-0"	6'-0"	1'-6"	51	76'-4 1/2"	42'-5 5/8"	9'-5 1/4"	94'-4 1/2"	93,998	82	82	16	82	3	1	11'-4"

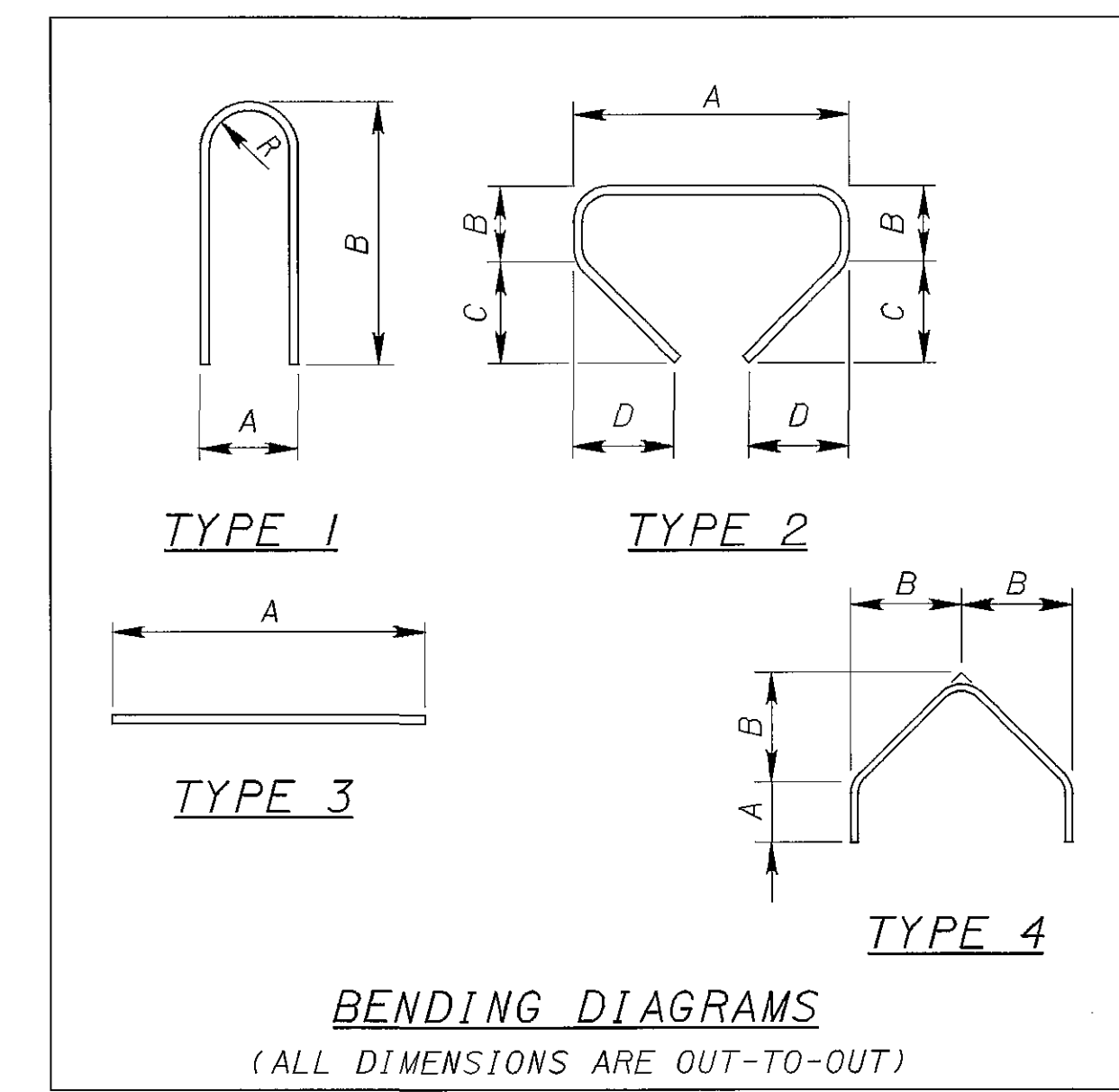
- NOTES:**
- FOR SECTION AT BEAM ENDS AND AT MIDSPAN, SEE SHEET [30/55].
 - FOR NUMBER OF STRANDS PER ROW, SEE SHEET [30/55].
 - FOR DETAILS NOT SHOWN, REFER TO O.D.O.T. STANDARD DRAWING PISD-1-99.



MODIFIED AASHTO TYPE 4 (72")
(AT BEAM ENDS)



MODIFIED AASHTO TYPE 4 (72")
(AT MID-SPAN)



BENDING DIAGRAMS
(ALL DIMENSIONS ARE OUT-TO-OUT)

MARK	TYPE	DIMENSIONS				
		A	B	C	D	R
F401	1	5 1/2"	6' - 4"			2 1/4"
F402	2	1' - 11 1/2"	6 1/4"	8 1/2"	8 1/2"	
F403	4	6 1/4"	11 3/4"			
F404	3	2' - 8"				
F501	3	30' - 0"				
F502	3	*				

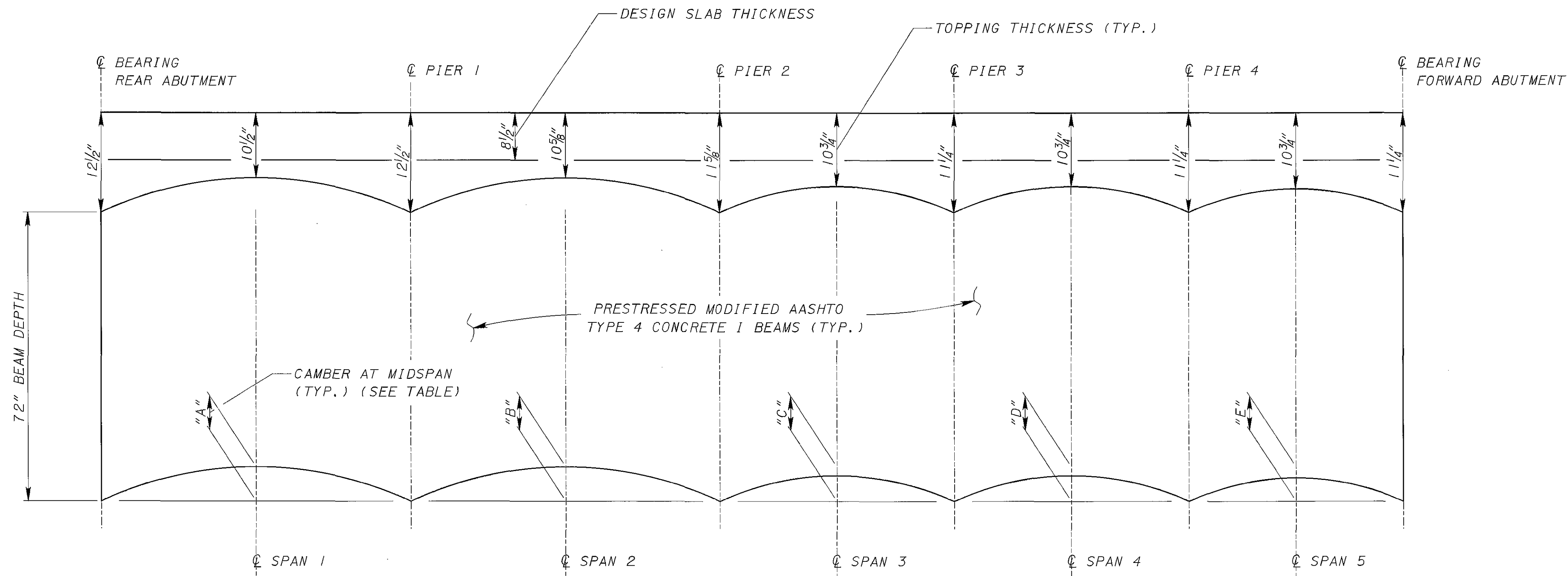
* - SEE TABLE OF BEAM DIMENSIONS AND REINFORCEMENT FOR BAR LENGTH, SHEET 29/55
 BAR SIZE IS INDICATED IN THE BAR MARK. THE FIRST LETTER IDENTIFIES THE BAR LOCATION, THE NEXT DIGIT INDICATES THE INCH-POUND BAR SIZE AND THE REMAINING DIGITS ITS SEQUENCE NUMBER. ALL STEEL SHALL BE EPOXY COATED.

	SPAN	BEAM NUMBER	NUMBER OF STRANDS PER ROW												CONCRETE STRENGTH (PSI)												
			END OF BEAM						MIDSPAN																		
			ROW NUMBER														TOTAL NUMBER OF STRANDS										
LEFT BRIDGE	1	(B1) THRU (B5)	11	11	8	8	6	4	2	3	3	3	3	3	65	18' - 0"	5	11	11	11	11	9	7	5	65	5000	7000
	1	(B6) THRU (B12)	11	11	8	8	6	-	-	3	3	3	3	2	58	18' - 0"	5	11	11	11	11	9	3	2	58	5000	7000
	2	(B13) THRU (B15)	11	11	8	8	6	4	2	3	3	3	3	3	65	18' - 0"	5	11	11	11	11	9	7	5	65	5000	7000
	2	(B16) THRU (B22)	11	11	8	8	6	-	-	3	3	3	3	2	58	18' - 0"	5	11	11	11	11	9	3	2	58	5000	7000
	3	(B23) THRU (B31)	11	8	4	-	-	-	-	-	-	-	3	3	29	-	-	11	11	7	-	-	-	-	29	5000	7000
RIGHT BRIDGE	1	(B50) THRU (B59)	11	11	8	8	6	-	-	3	3	3	3	2	58	18' - 0"	5	11	11	11	11	9	3	2	58	5000	7000
	2	(B60) THRU (B66)	11	11	8	8	6	-	-	3	3	3	3	2	58	18' - 0"	5	11	11	11	11	9	3	2	58	5000	7000
	2	(B67) THRU (B69)	11	8	8	8	-	-	-	-	3	3	3	3	47	-	-	11	11	11	11	3	-	-	47	5000	7000
	3	(B70) THRU (B78)	11	8	4	-	-	-	-	-	-	-	3	3	29	-	-	11	11	7	-	-	-	-	29	5000	7000
	4	(B79) THRU (B87)	11	8	4	-	-	-	-	-	-	-	3	3	29	-	-	11	11	7	-	-	-	-	29	5000	7000
5	(B88) THRU (B96)	11	11	-	-	-	-	-	-	-	-	-	3	25	-	-	11	11	3	-	-	-	-	25	5000	7000	

NOTES:

- FOR BEAM ELEVATION AND BEAM DIMENSIONS, SEE SHEET 29/55.
- BEAM AGE AT PLACEMENT OF PIER DIAPHRAGMS SHALL BE A MINIMUM OF 90 DAYS.
- INITIAL PRESTRESSING LOAD PER STRAND = 33,318 LBS

- SHIPPING STRANDS: THE FABRICATOR MAY ADD SHIPPING STRANDS AT THE LOCATIONS SHOWN ON SHEET 1 OF 8 OF ODOT STANDARD DRAWING PSID 1-99. THESE SHIPPING STRANDS OR ANY ADDITIONAL SHIPPING STRANDS SHALL BE DEBONDED FOR THE ENTIRE LENGTH OF THE BEAM EXCEPT FOR THE LAST 10' - 0" AT EACH END. THE STRANDS SHALL BE CUT AFTER HANDLING OPERATIONS ARE COMPLETE.



BEAM CAMBER AND DECK THICKNESS DIAGRAM

BEAM CAMBERS AT MIDSPAN

	SPAN 1 "A"			SPAN 2 "B"				SPAN 3 "C"		SPAN 4 "D"		SPAN 5 "E"	
	(B1) THRU (B5)	(B6) THRU (B12)	(B50) THRU (B59)	(B13) THRU (B15)	(B16) THRU (B22)	(B60) THRU (B66)	(B67) THRU (B69)	(B23) THRU (B31)	(B70) THRU (B78)	(B32) THRU (B40)	(B79) THRU (B87)	(B41) THRU (B49)	(B88) THRU (B96)
CAMBER AT TIME OF RELEASE DUE TO PRESTRESSING	5 ⁵ / ₁₆ "	5 ⁹ / ₁₆ "	5 ³ / ₈ "	5 ³ / ₄ "	5 ⁷ / ₁₆ "	5 ⁷ / ₁₆ "	4 ⁹ / ₁₆ "	1 ¹³ / ₁₆ "	1 ¹³ / ₁₆ "	1 ¹³ / ₁₆ "	1 ¹³ / ₁₆ "	1 ⁷ / ₁₆ "	1 ⁷ / ₁₆ "
DEFLECTION DUE TO SELF WEIGHT	2 ¹⁵ / ₁₆ "	2 ¹⁵ / ₁₆ "	2 ¹⁵ / ₁₆ "	2 ⁷ / ₈ "	2 ¹³ / ₁₆ "	2 ¹³ / ₁₆ "	2 ¹³ / ₁₆ "	1 ⁵ / ₁₆ "	1 ⁵ / ₁₆ "	1 ⁵ / ₁₆ "	1 ⁵ / ₁₆ "	1 ¹ / ₁₆ "	1 ¹ / ₁₆ "
CAMBER AT TIME OF RELEASE (NET)	3"	2 ⁵ / ₈ "	2 ⁷ / ₁₆ "	2 ⁷ / ₈ "	2 ⁵ / ₈ "	2 ⁵ / ₈ "	1 ¹¹ / ₁₆ "	3 ³ / ₄ "	3 ³ / ₄ "	3 ³ / ₄ "	3 ³ / ₄ "	3 ³ / ₄ "	3 ³ / ₄ "
CAMBER AT TIME OF ERECTION	5 ¹ / ₄ "	4 ⁹ / ₁₆ "	4 ¹ / ₄ "	5 ¹ / ₁₆ "	4 ⁹ / ₁₆ "	4 ⁹ / ₁₆ "	2 ¹⁵ / ₁₆ "	1 ⁹ / ₁₆ "	1 ⁹ / ₁₆ "	1 ⁹ / ₁₆ "	1 ⁹ / ₁₆ "	1 ⁵ / ₁₆ "	1 ⁵ / ₁₆ "
LONG TERM CAMBER	7 ¹ / ₂ "	6 ⁹ / ₁₆ "	6 ¹ / ₈ "	7 ¹ / ₄ "	6 ⁹ / ₁₆ "	6 ⁹ / ₁₆ "	4 ⁵ / ₁₆ "	2 ¹ / ₄ "	2 ¹ / ₄ "	2 ¹ / ₄ "	2 ¹ / ₄ "	1 ⁷ / ₈ "	1 ⁷ / ₈ "

NOTES:

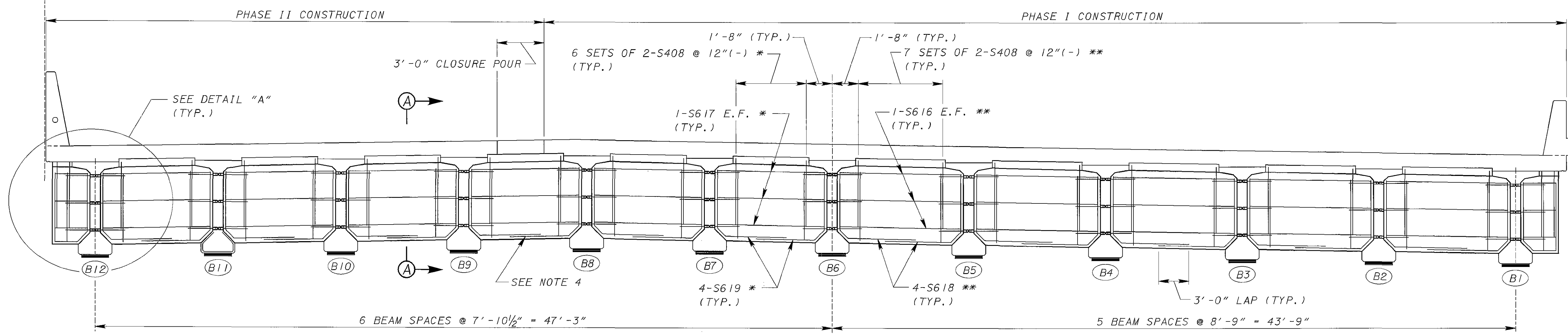
- DECK SLAB THICKNESS FOR CONCRETE QUANTITY:
 THE TOPPING THICKNESSES SHOWN FROM THE TOP OF THE DECK SLAB TO THE TOP OF THE TOP FLANGE ALONG THE CENTERLINE OF THE I-BEAM ARE THEORETICAL DIMENSIONS. THE HAUNCH DEPTH IS THE TOPPING THICKNESS MINUS THE DESIGN SLAB THICKNESS. THE DEPARTMENT WILL PAY FOR SUPERSTRUCTURE CONCRETE BASED ON THE DESIGN SLAB THICKNESS AND THE AVERAGE OF THE THEORETICAL HAUNCH DEPTHS AT MID-SPAN AND AT EACH BEAM BEARING EVEN THOUGH THE DEVIATION FROM THE DIMENSIONS SHOWN MAY BE NECESSARY TO PLACE THE DECK SURFACE AT THE FINISHED GRADE. ONCE ALL THE BEAMS ARE SET IN THEIR FINAL POSITION, THE ACTUAL CAMBER FOR EACH MEMBER WILL BE THE TOP OF THE BEAM ELEVATION AT MID-SPAN MINUS THE AVERAGE TOP OF BEAM ELEVATION AT EACH BEARING. THE ACTUAL TOPPING THICKNESS AT MID-SPAN WILL BE THE THEORETICAL DIMENSION PLUS OR MINUS THE DIFFERENCE BETWEEN THE ACTUAL AND ANTICIPATED CAMBER.
- FOR SCREED ELEVATIONS, SEE SHEETS [40/55] THRU [46/55].
- FOR BEAM DETAILS, SEE SHEETS [29/55] AND [30/55].

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:32 PM
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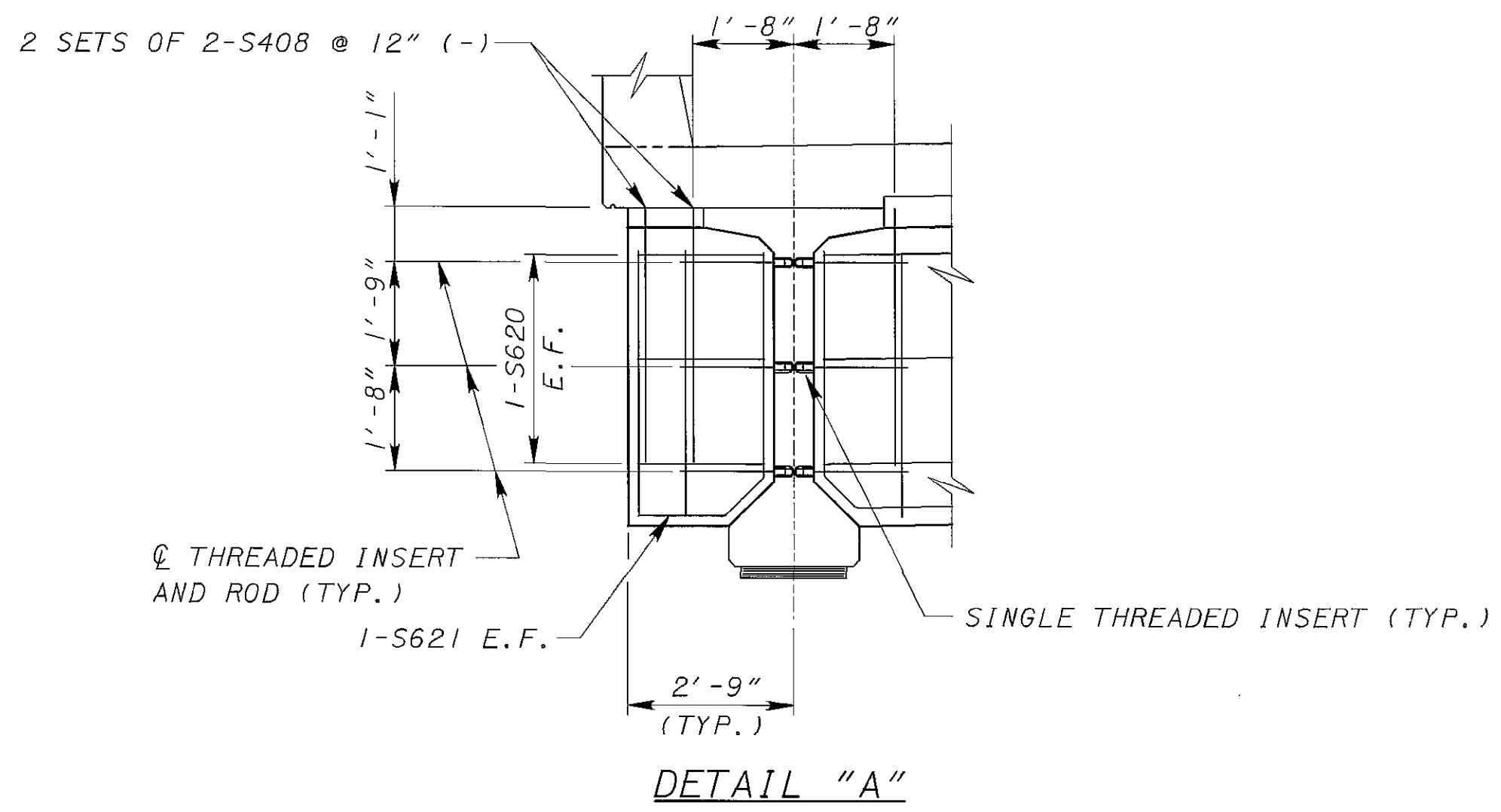
BRIDGE NO. MOT-75-1523
 IR-75 OVER THE GREAT MIAMI RIVER
 BEAM CAMBER AND DECK THICKNESS
 (PRESTRESSED CONCRETE OPTION)

MOT-75-14.60
 PID 23828

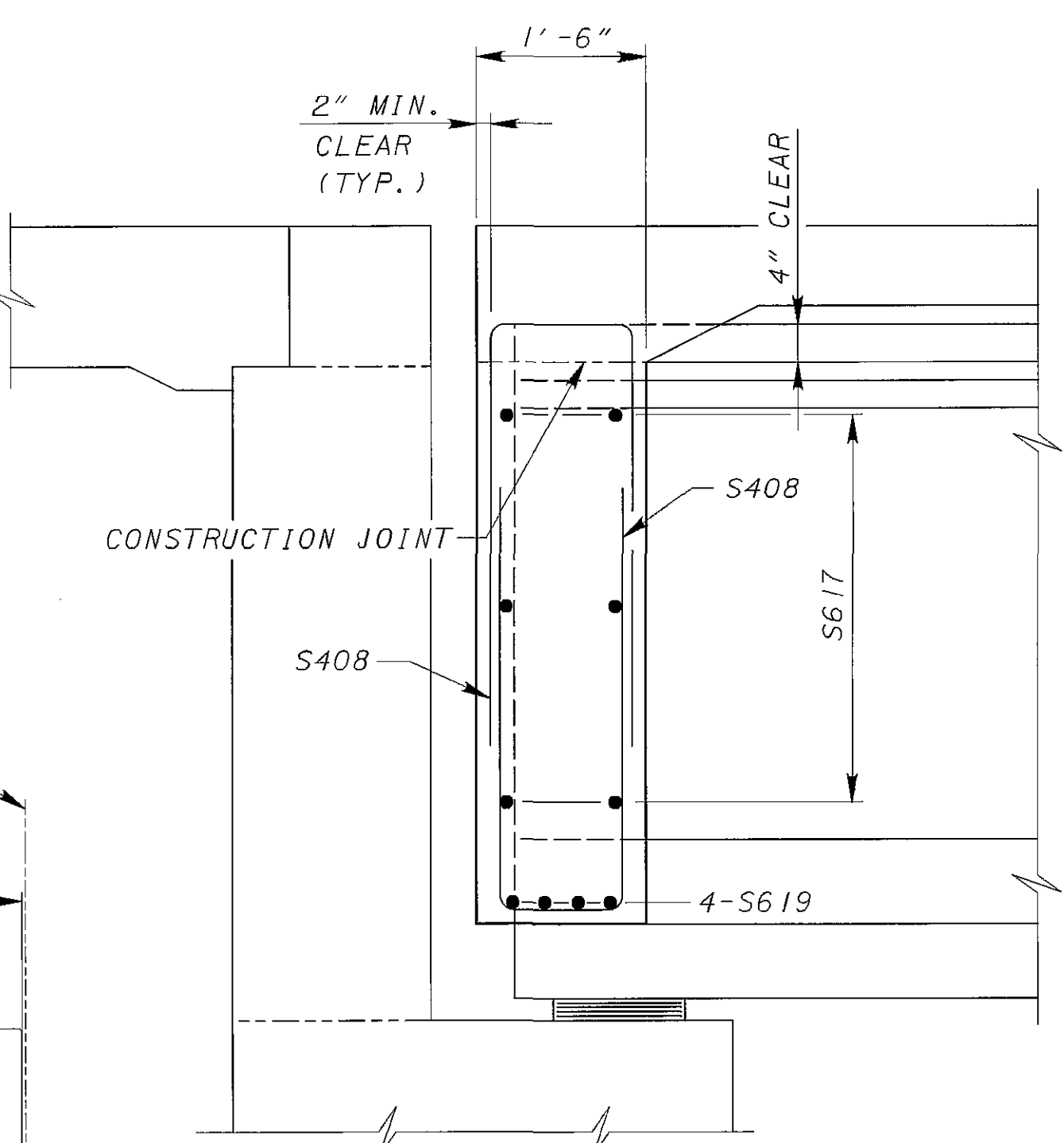
☉ SURVEY AND CONSTRUCTION
 I.R. 75



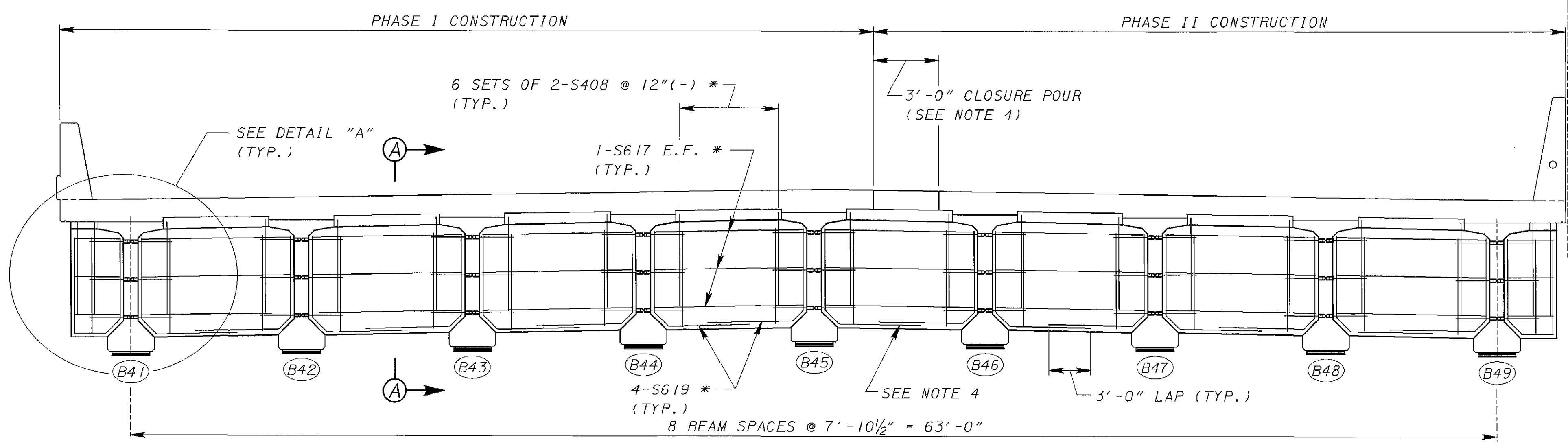
**ELEVATION
 REAR ABUTMENT**



DETAIL "A"



**SECTION A-A
 (EXPANSION JOINT NOT SHOWN)**



**ELEVATION
 FORWARD ABUTMENT**

LEGEND:

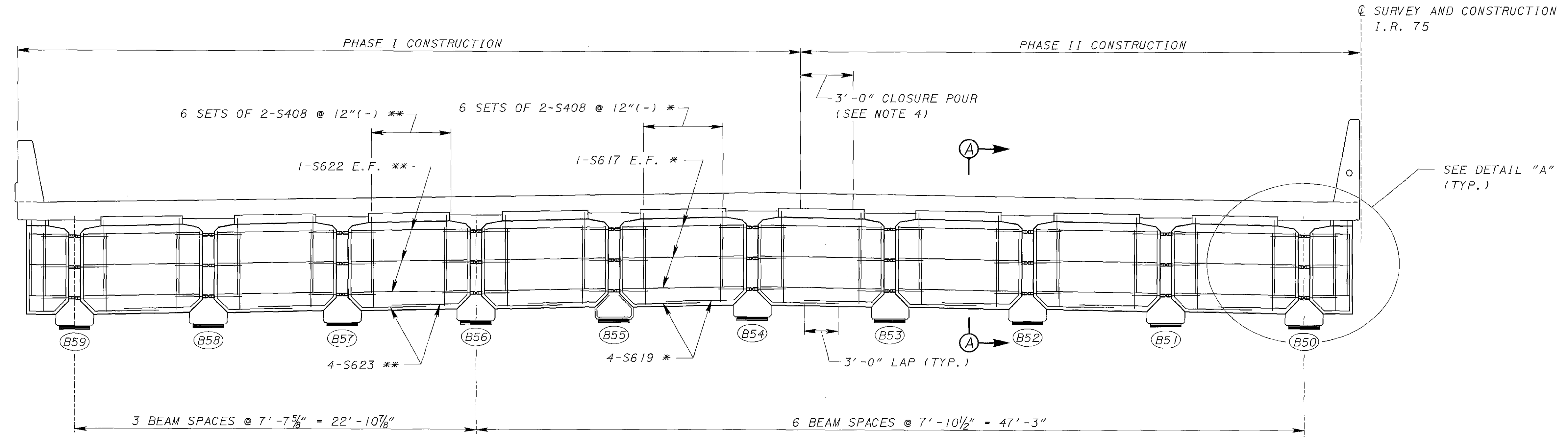
- * - TYPICAL BETWEEN BEAMS (B6) THRU (B12) AND BEAMS (B4) THRU (B49)
- ** - TYPICAL BETWEEN BEAMS (B1) THRU (B6)
- (BXX) - BEAM NUMBER
- E.F. - EACH FACE

NOTES:

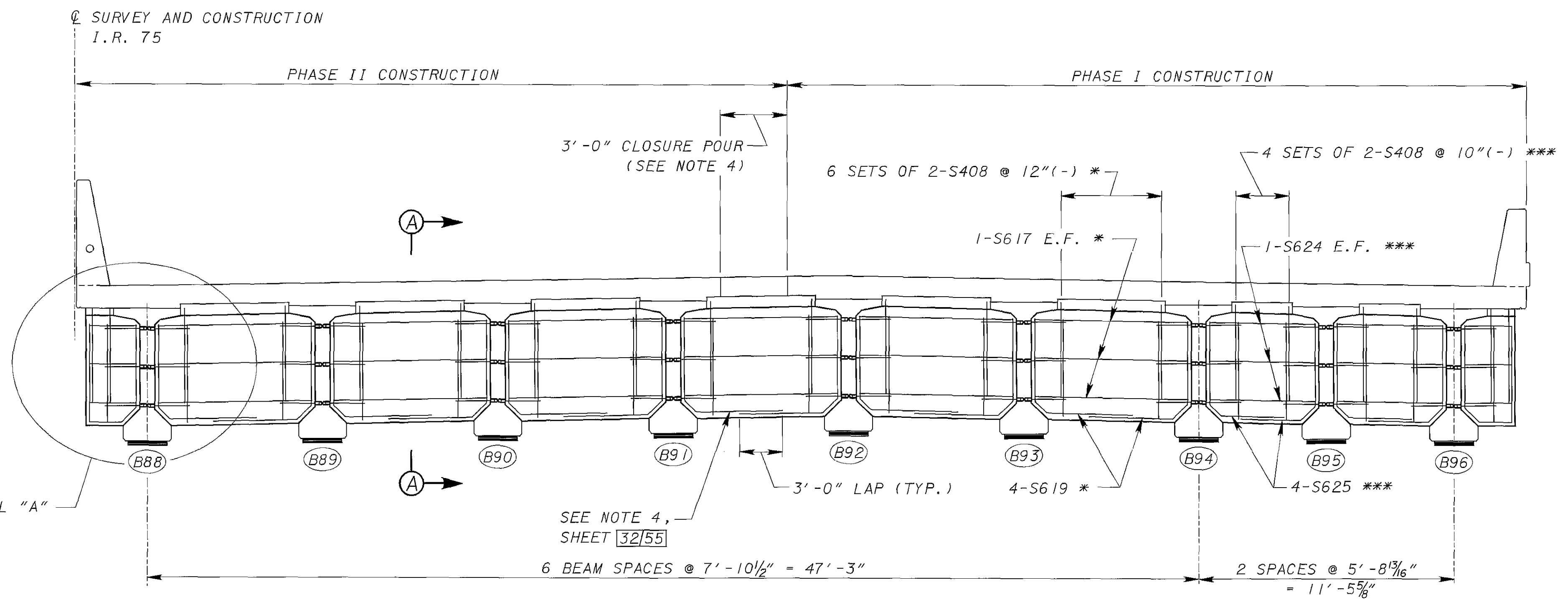
1. FOR BEAM SPACING SEE FRAMING PLAN, SHEET [27/55].
2. FOR REINFORCING STEEL LIST, SEE SHEET [55/55].
3. FOR DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING PISD-1-99.
4. DIAPHRAGMS AT CLOSURE POUR TO BE CAST DURING PHASE II CONSTRUCTION.

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Plotted By: ecmack Date: 7/21/2007 Time: 3:42:27 PM
 Filename: g:\c004\0066\brldge\concrete.alt\cadd\23828scd0.dgn



ELEVATION
REAR ABUTMENT



ELEVATION
FORWARD ABUTMENT

LEGEND:

- * - TYPICAL BETWEEN BEAMS (B50) THRU (B56) AND BEAMS (B88) THRU (B94)
- ** - TYPICAL BETWEEN BEAMS (B56) THRU (B59)
- *** - TYPICAL BETWEEN BEAMS (B94) THRU (B96)
- (BXX) - BEAM NUMBER

NOTES:

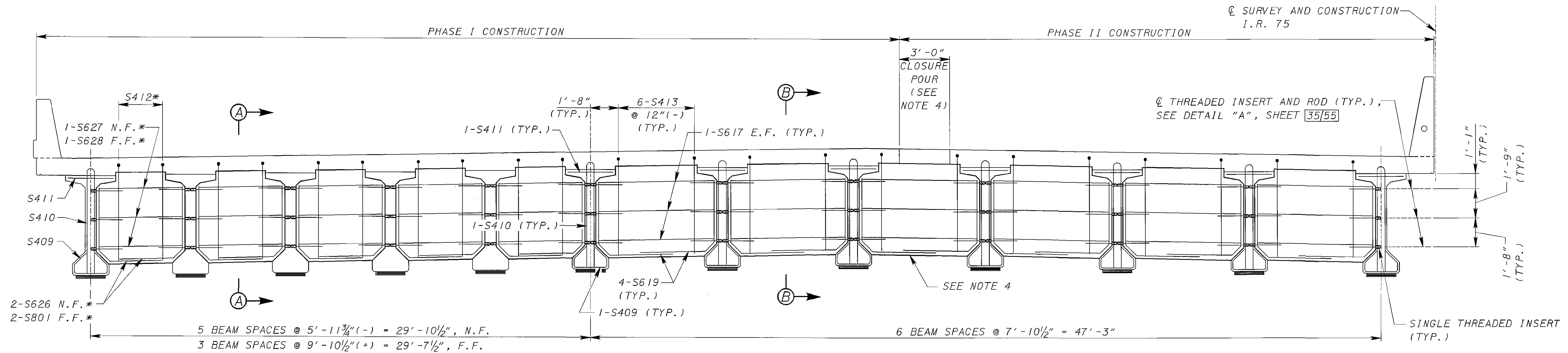
1. FOR BEAM SPACING SEE FRAMING PLAN SHEET 32/55.
2. FOR REINFORCING STEEL LIST, SEE SHEET 32/55.
3. FOR DETAILS NOT SHOWN, REFER TO STANDARD DRAWING PSID-1-99.
4. DIAPHRAGMS AT CLOSURE POUR TO BE CAST DURING PHASE II CONSTRUCTION
5. FOR SECTION A-A AND DETAIL A, SEE SHEET 32/55.

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I.R. 75

© SURVEY AND CONSTRUCTION
I.R. 75

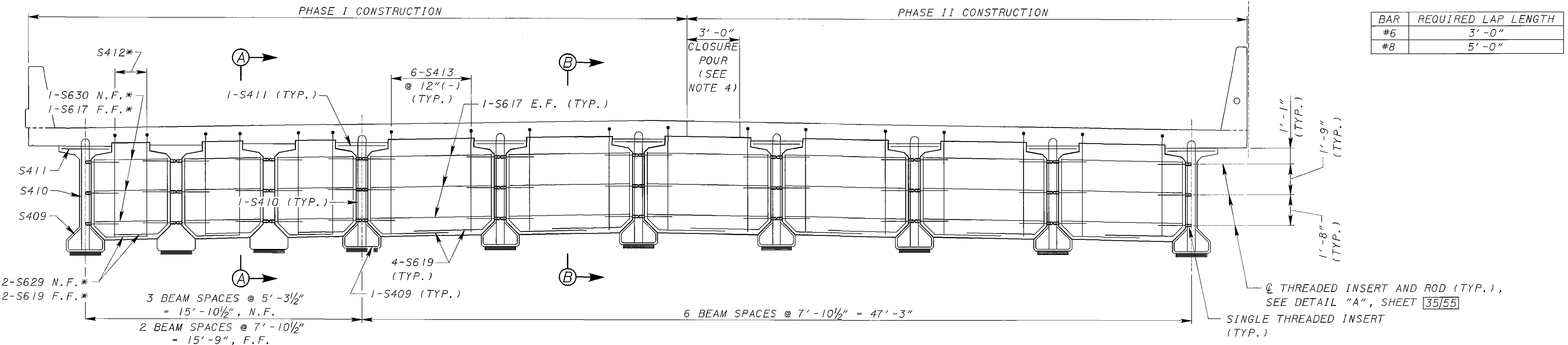
DESIGNED	JDH
CHECKED	NFF
DRAWN	CAG
REVIEWED	RER
DATE	5/27/05
STRUCTURE FILE NUMBER	5708710
END DIAPHRAGM DETAILS - RIGHT BRIDGE (PRESTRESSED CONCRETE OPTION) BRIDGE NO. MOT-75-1523 I.R. 75 OVER THE GREAT MIAMI RIVER	
MOT-75-14.60 PID 23828	
33	55
268 314	

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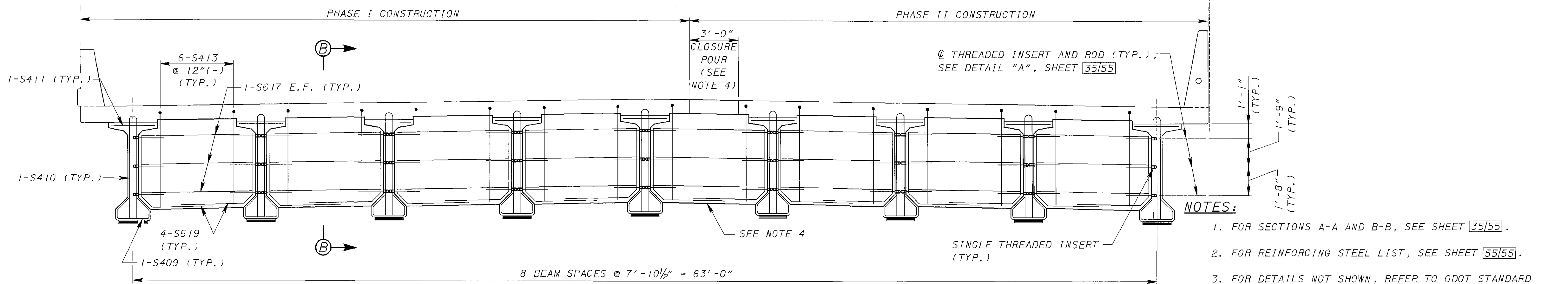
ELEVATION - PIER 1

* - FOR PLACEMENT OF REINFORCING STEEL WITHIN VARIABLE SPACED DIAPHRAGMS, SEE PARTIAL PLAN, SHEET 35/55



ELEVATION - PIER 2

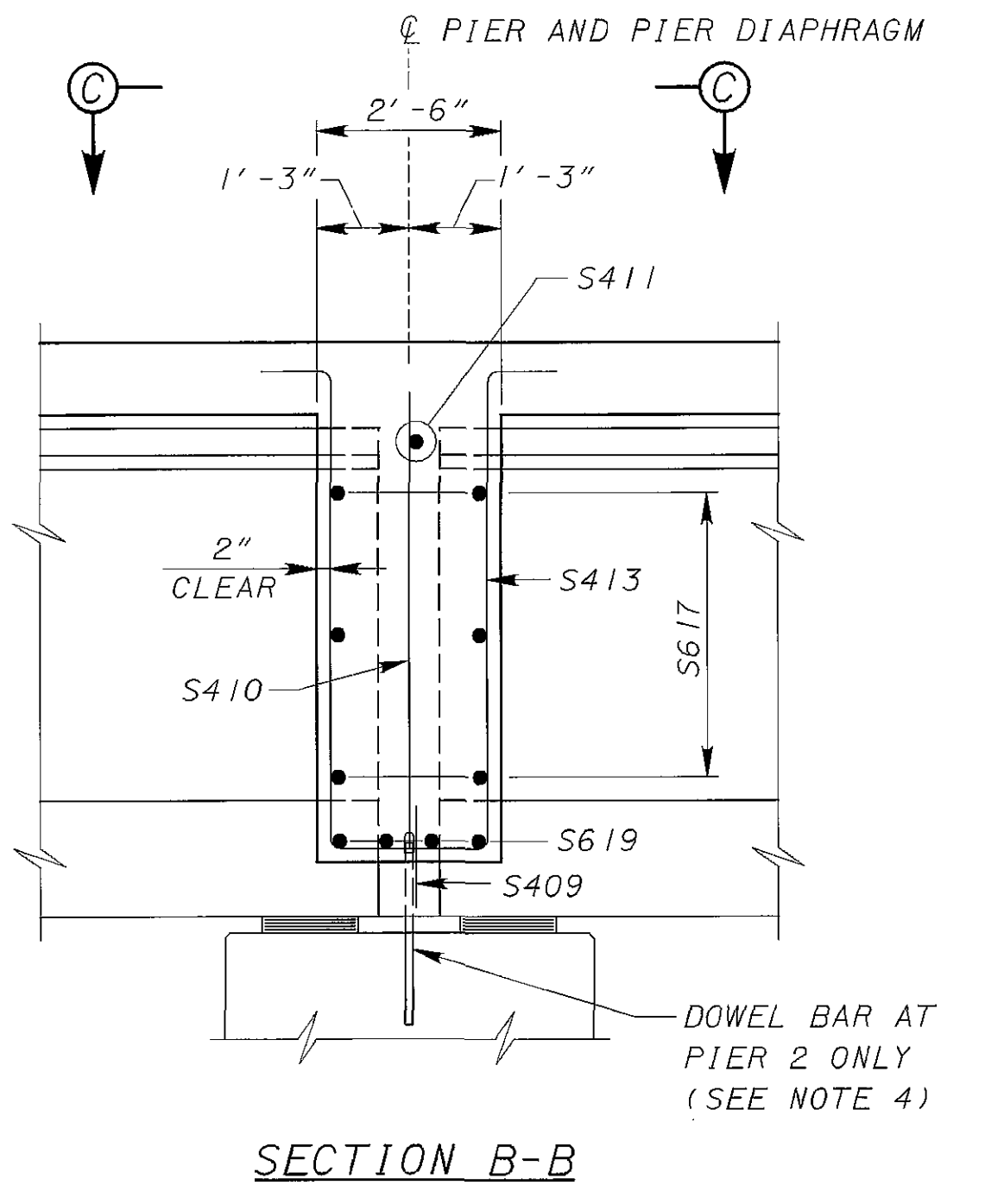
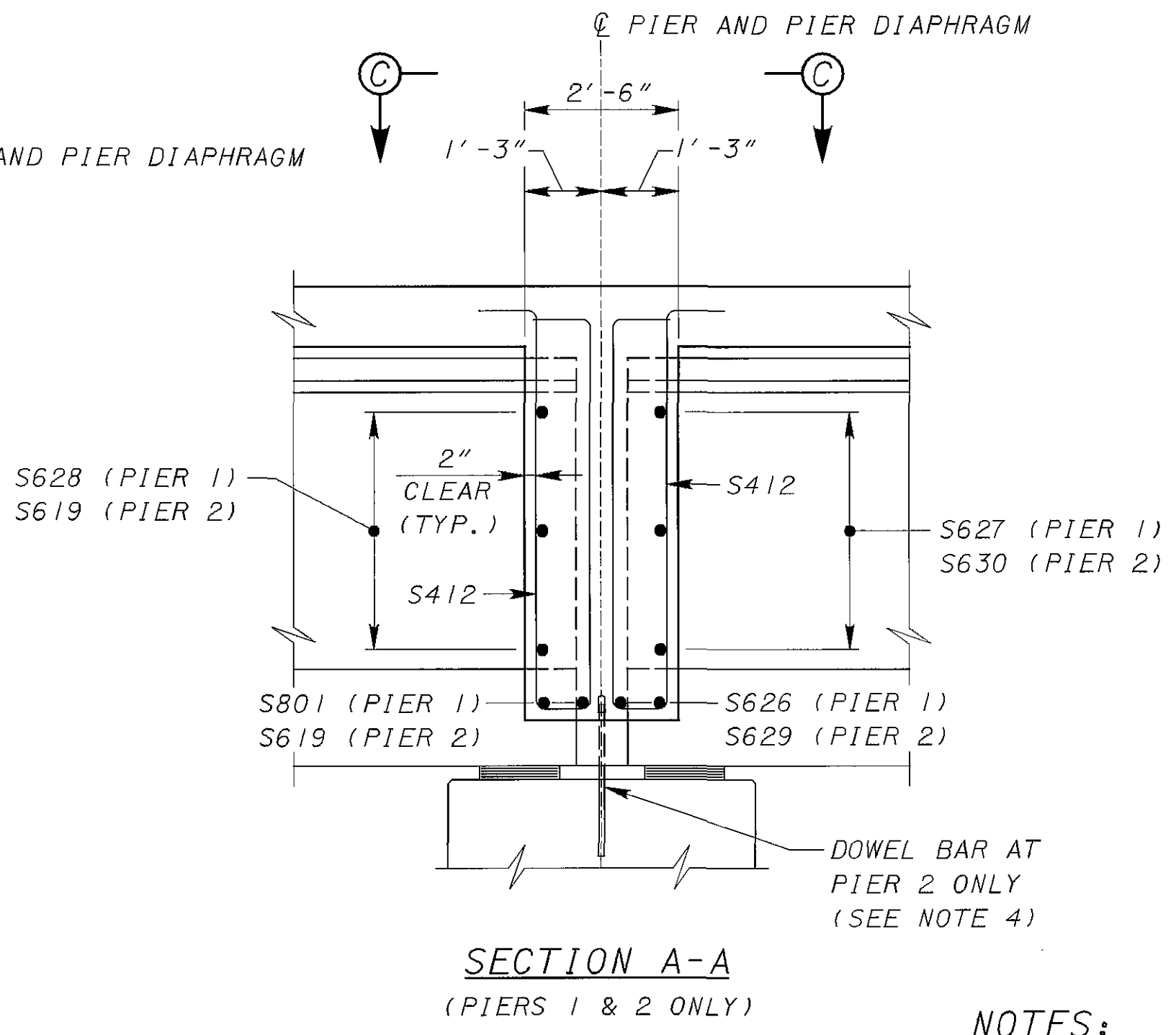
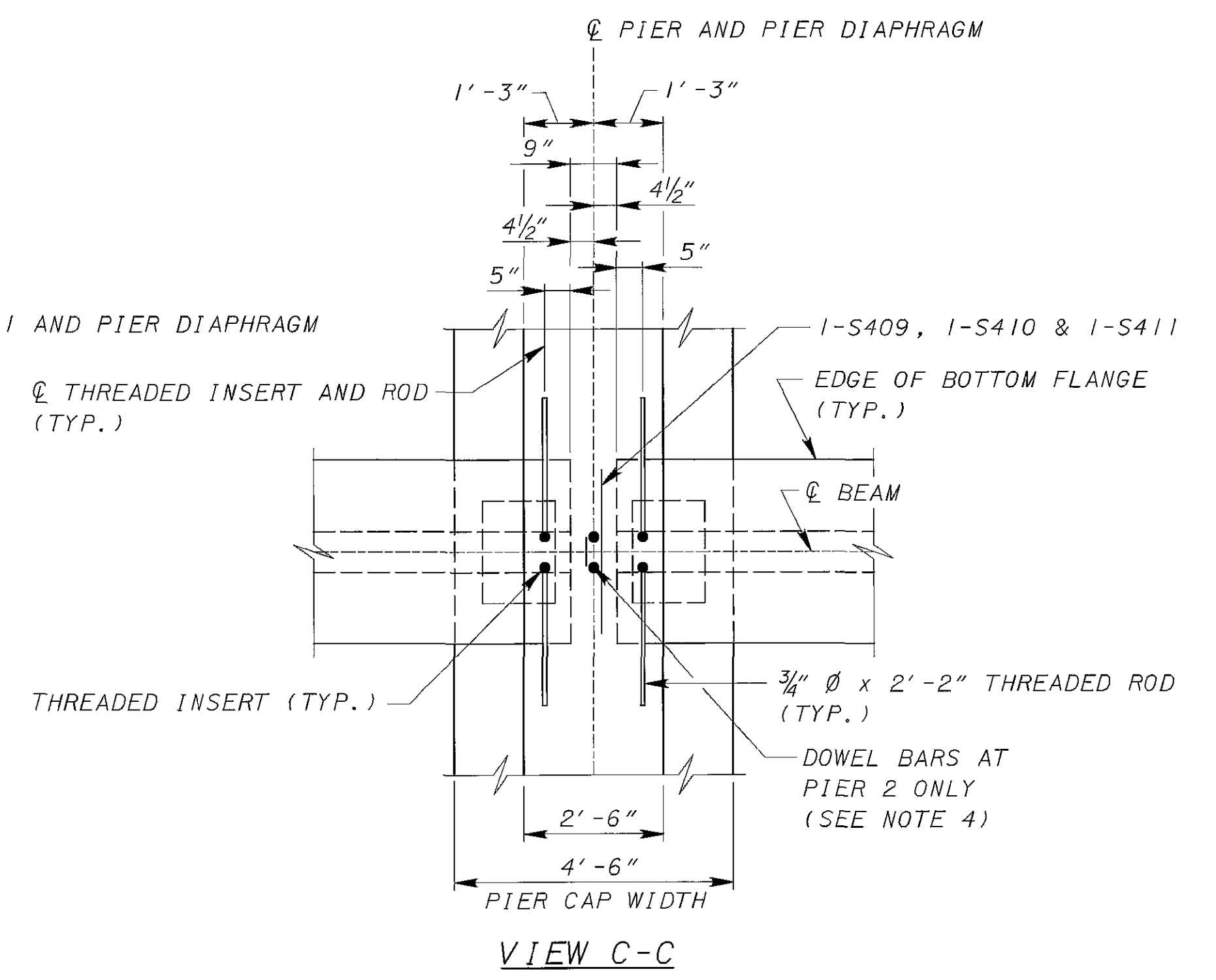
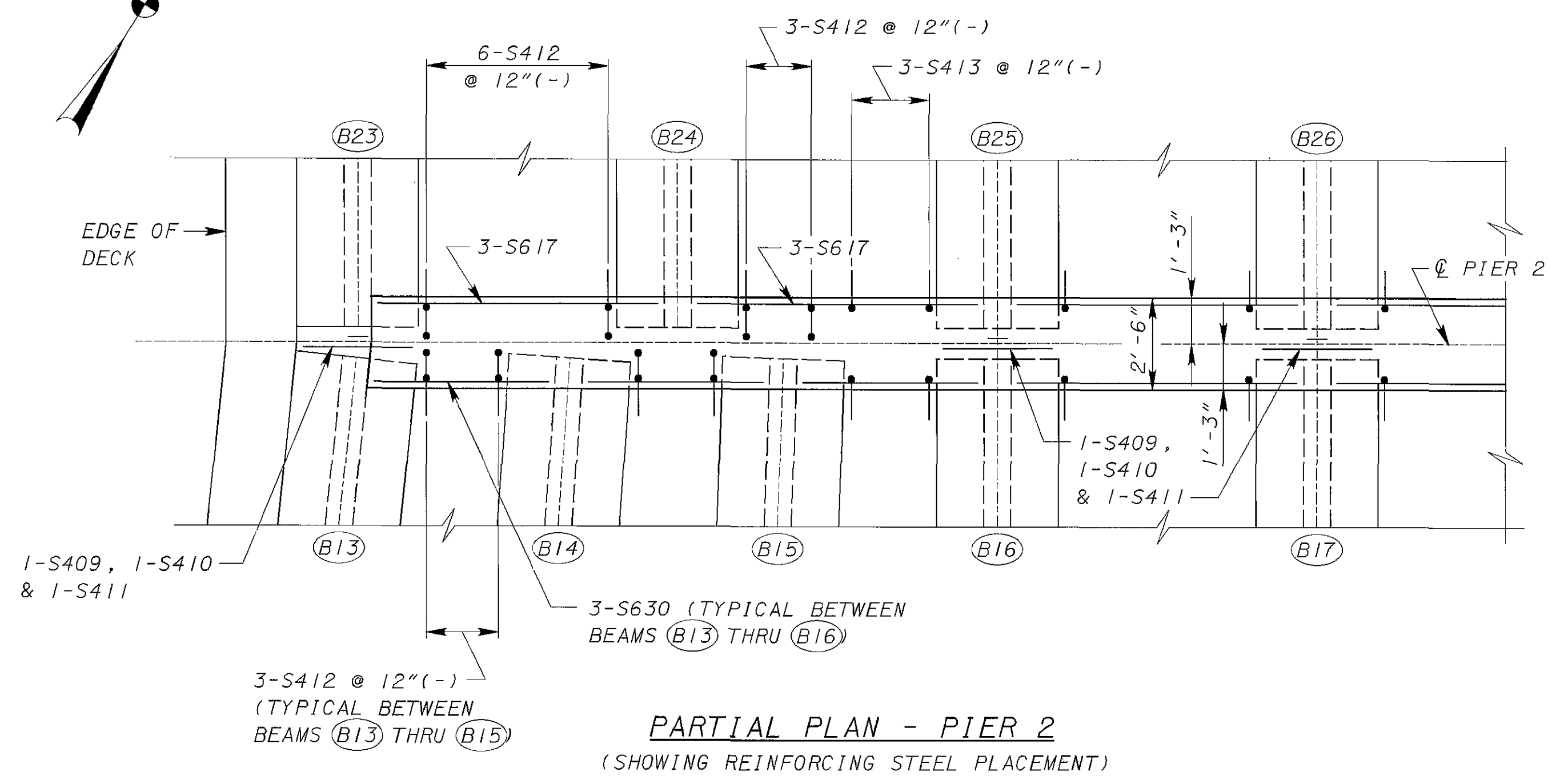
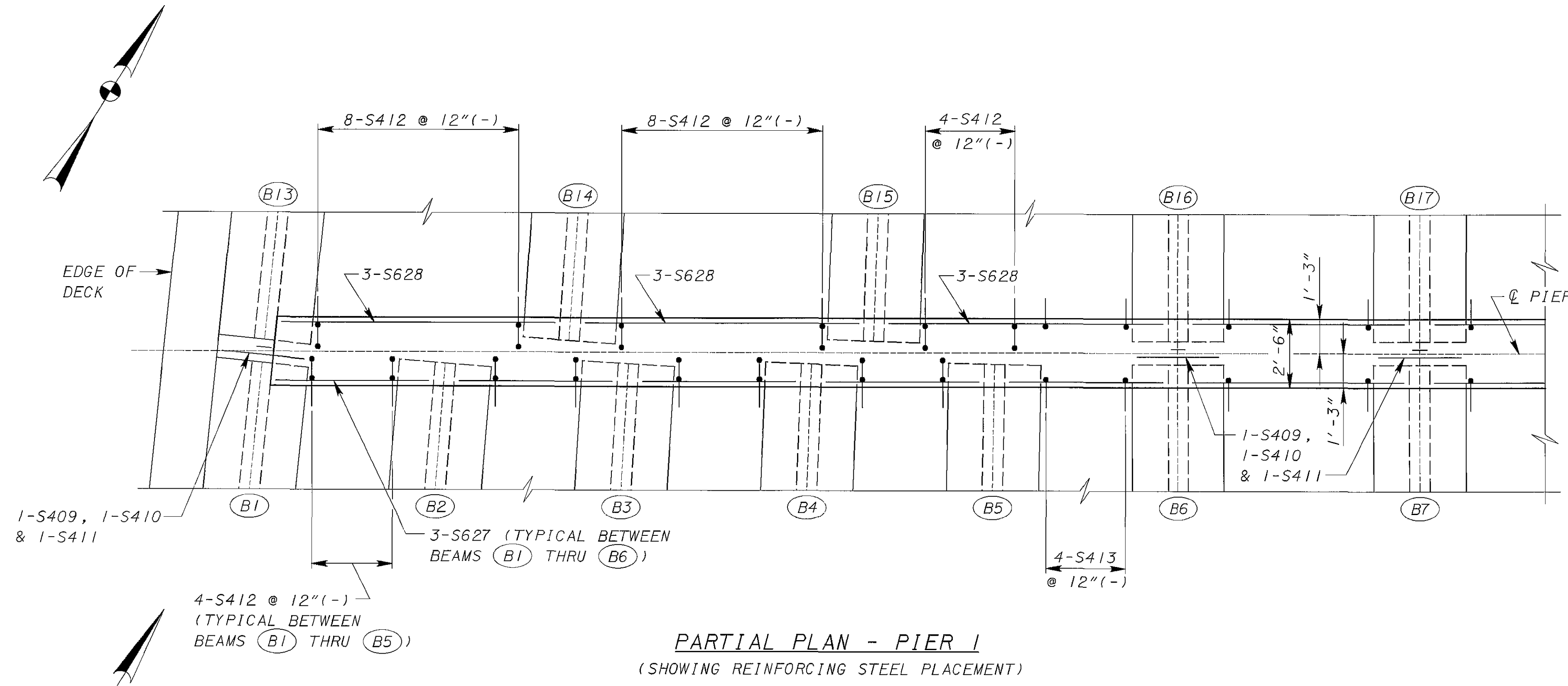
* - FOR PLACEMENT OF REINFORCING STEEL WITHIN VARIABLE SPACED DIAPHRAGMS, SEE PARTIAL PLAN, SHEET 35/55



ELEVATION - PIERS 3 AND 4

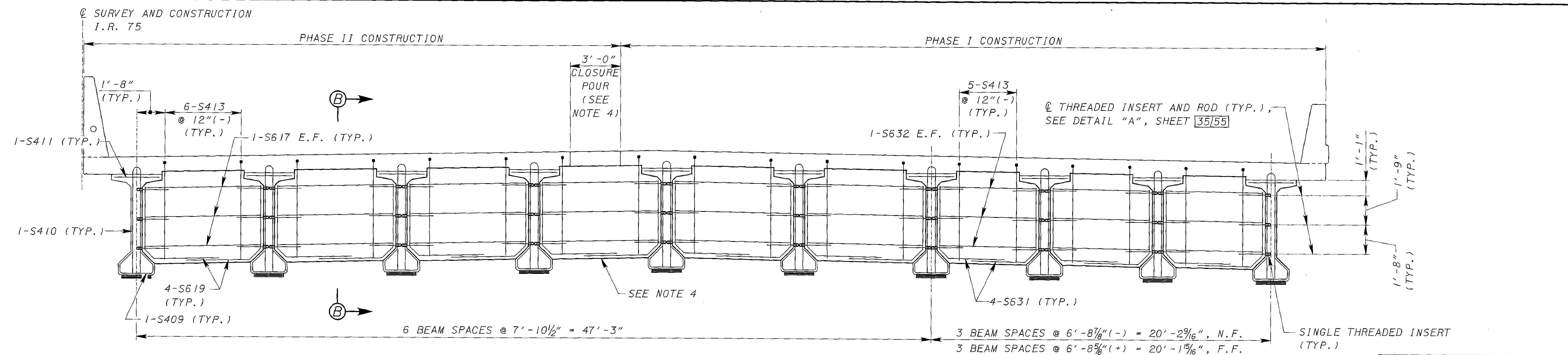
- NOTES:
1. FOR SECTIONS A-A AND B-B, SEE SHEET 35/55.
 2. FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
 3. FOR DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING PISD-1-99.
 4. DIAPHRAGMS AT CLOSURE POUR TO BE CAST DURING PHASE II CONSTRUCTION.

Plotted By: ecmack
 Date: 7/27/2007 Time: 3:42:23 PM
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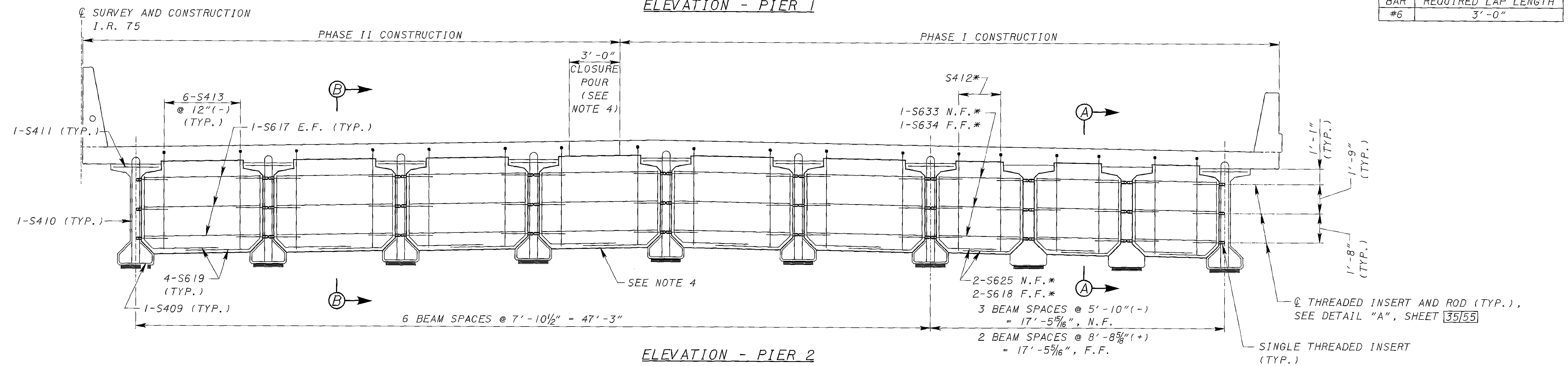
- NOTES:**
1. FOR PIER DIAPHRAGM ELEVATIONS, SHEET 34/55.
 2. FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
 3. FOR DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING PISD-1-99.
 4. FOR DOWEL BAR DETAILS, SEE BEARING DETAILS SHEET 49/55.

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:24 PM
 Filename: g:\co04\0066\bridge\concrete.alt\cadd\23828sd05.dgn



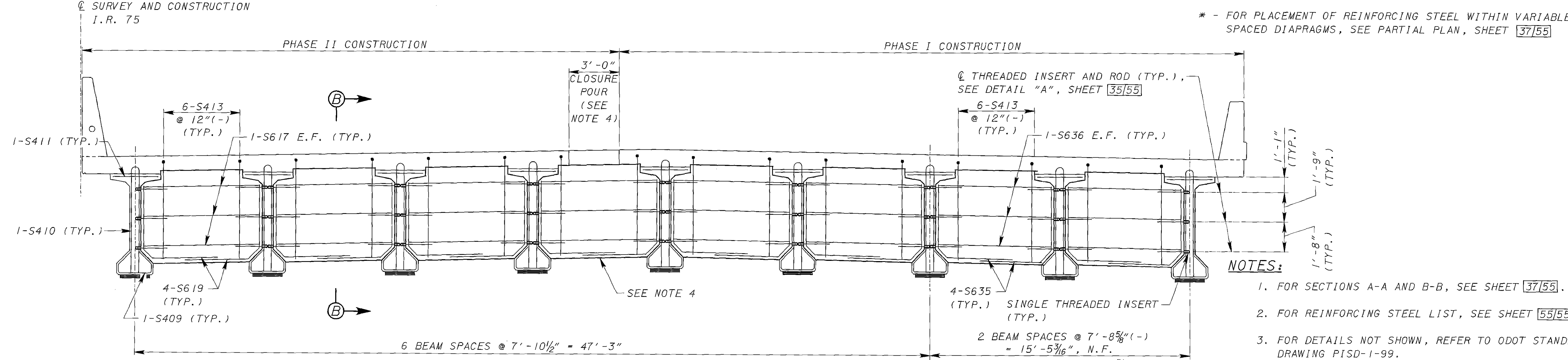
ELEVATION - PIER 1

BAR #	REQUIRED LAP LENGTH
#6	3'-0"



ELEVATION - PIER 2

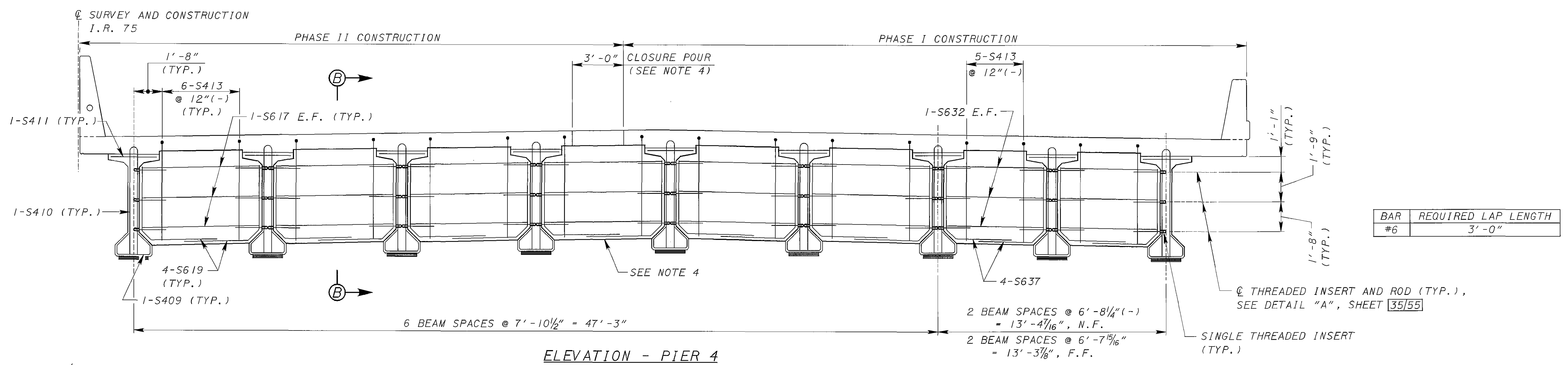
* - FOR PLACEMENT OF REINFORCING STEEL WITHIN VARIABLE SPACED DIAPHRAGMS, SEE PARTIAL PLAN, SHEET 37/55



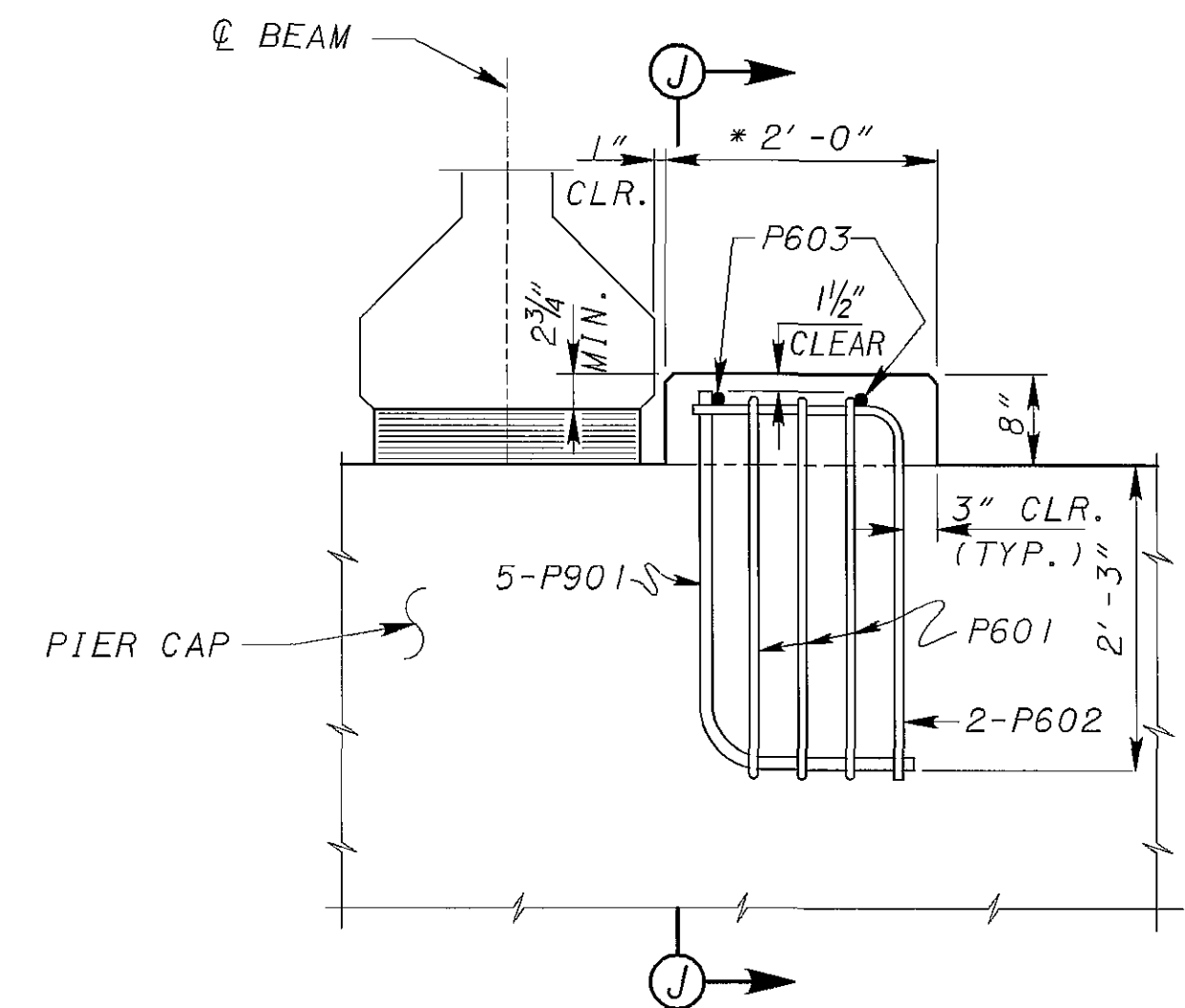
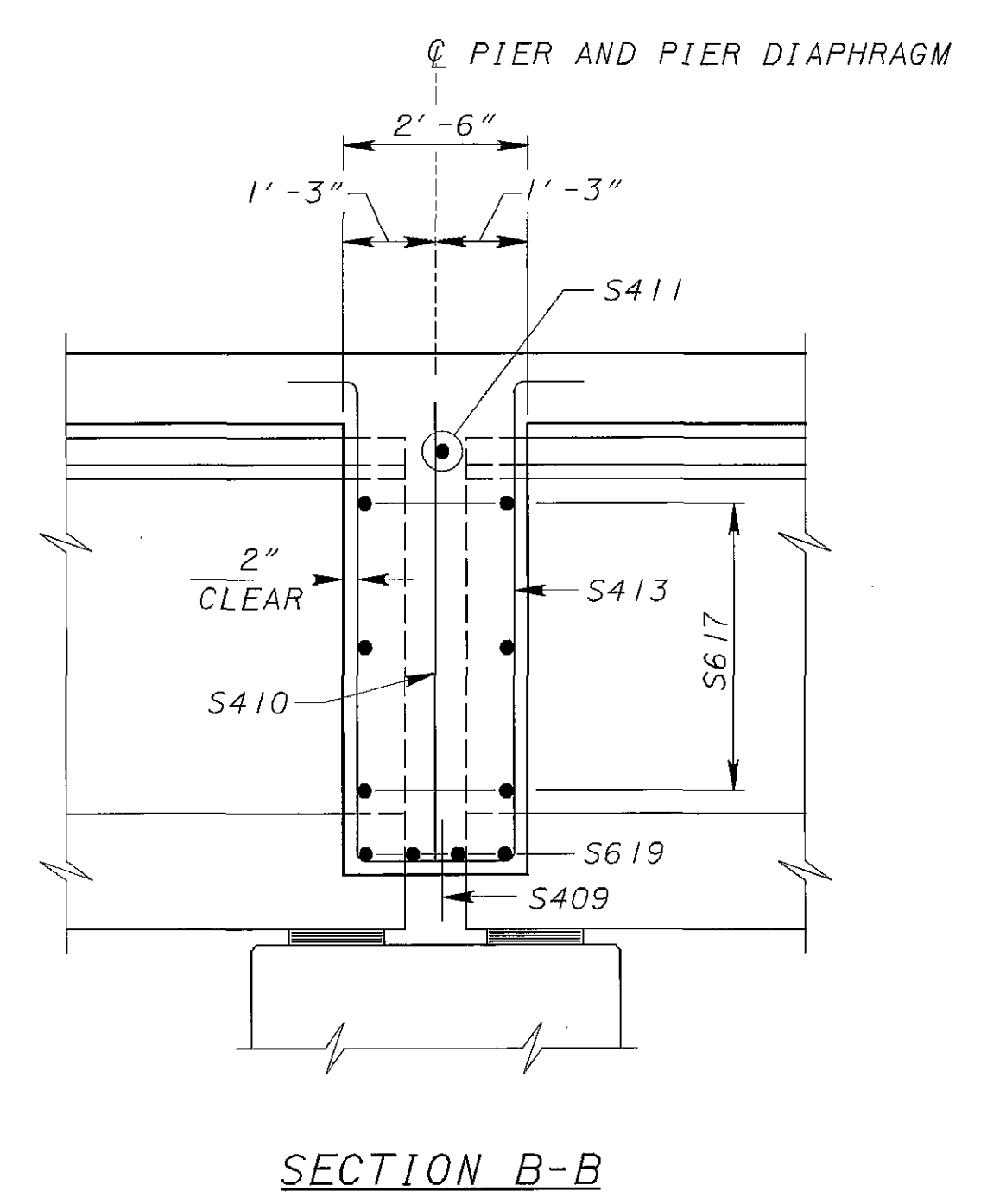
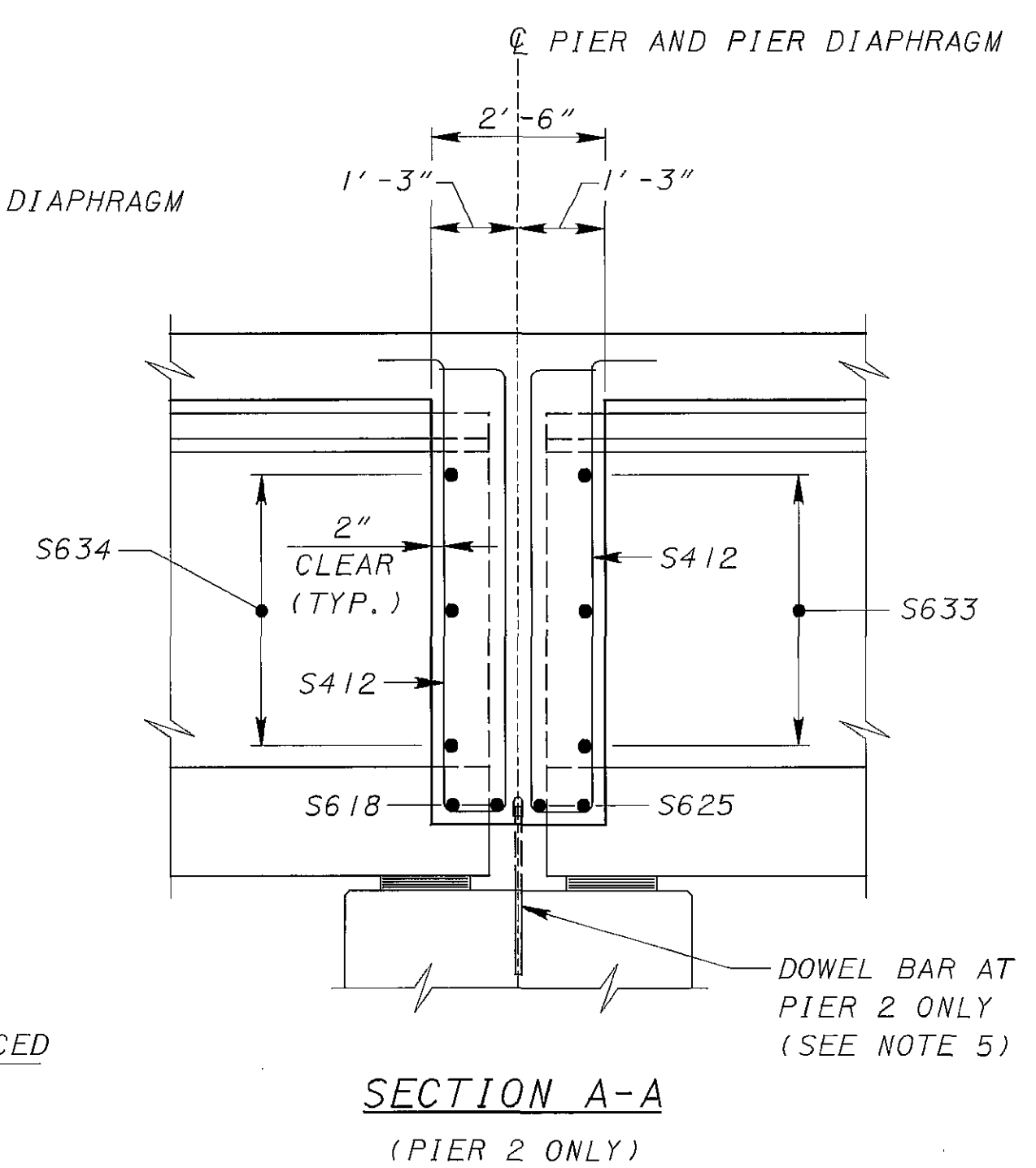
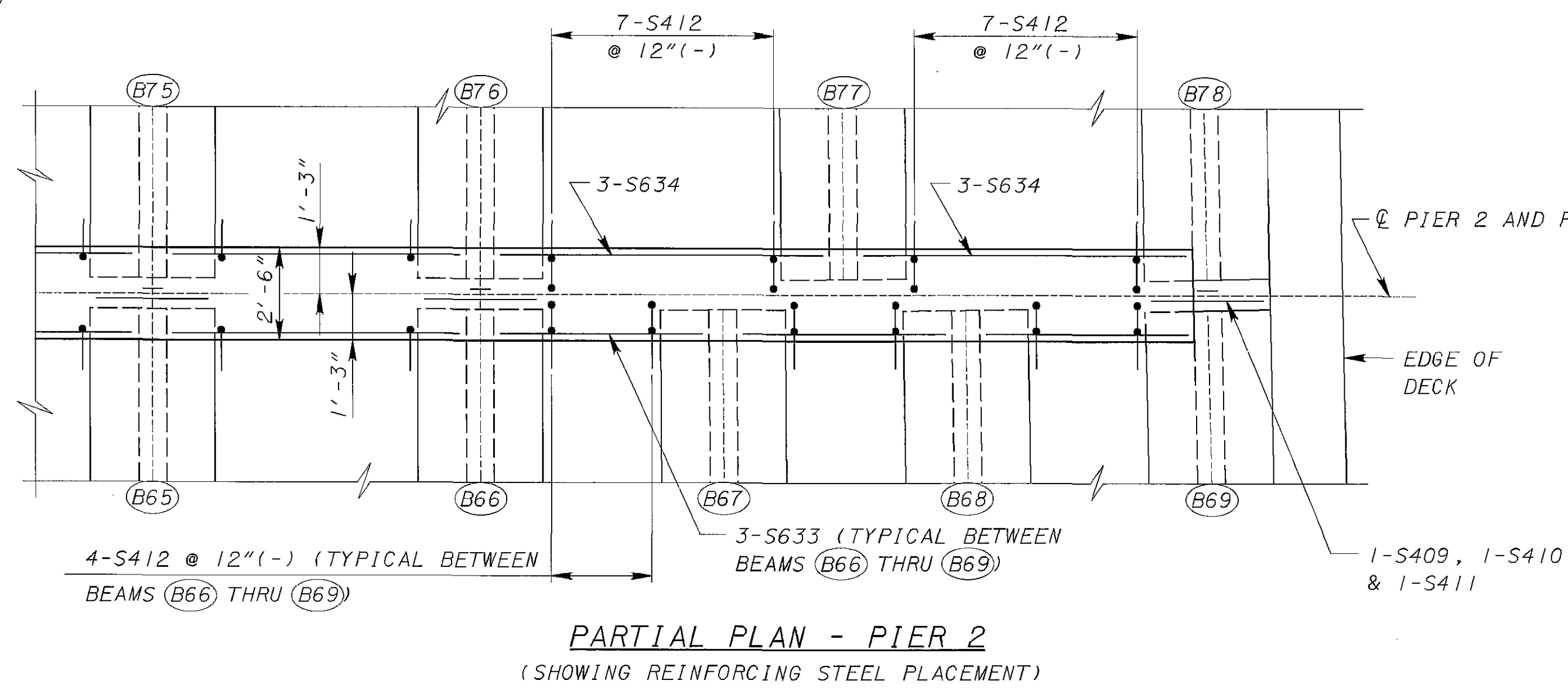
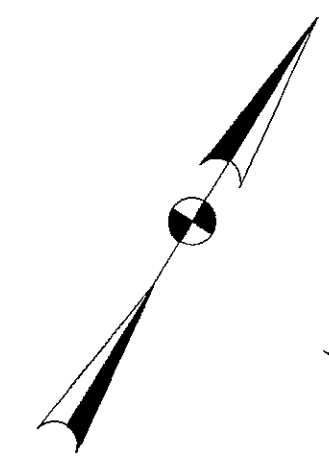
ELEVATION - PIER 3

- NOTES:
1. FOR SECTIONS A-A AND B-B, SEE SHEET 37/55.
 2. FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
 3. FOR DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING PISD-1-99.
 4. DIAPHRAGMS AT CLOSURE POUR TO BE CAST DURING PHASE II CONSTRUCTION.

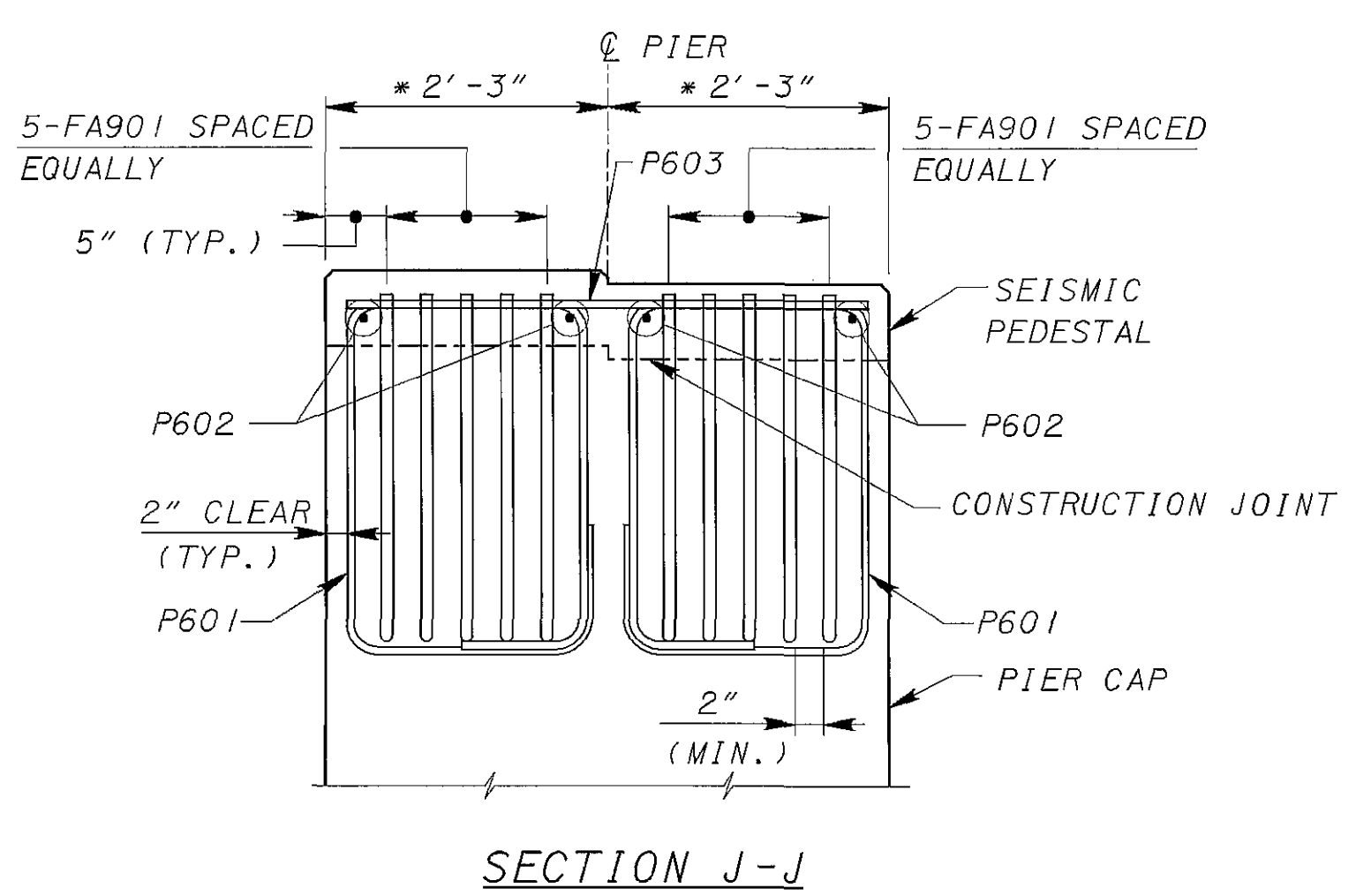
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 Filename: g:\co04\0066\brldge\concrete.alt\cadd\23828sc06.dgn



BAR	REQUIRED LAP LENGTH
#6	3'-0"



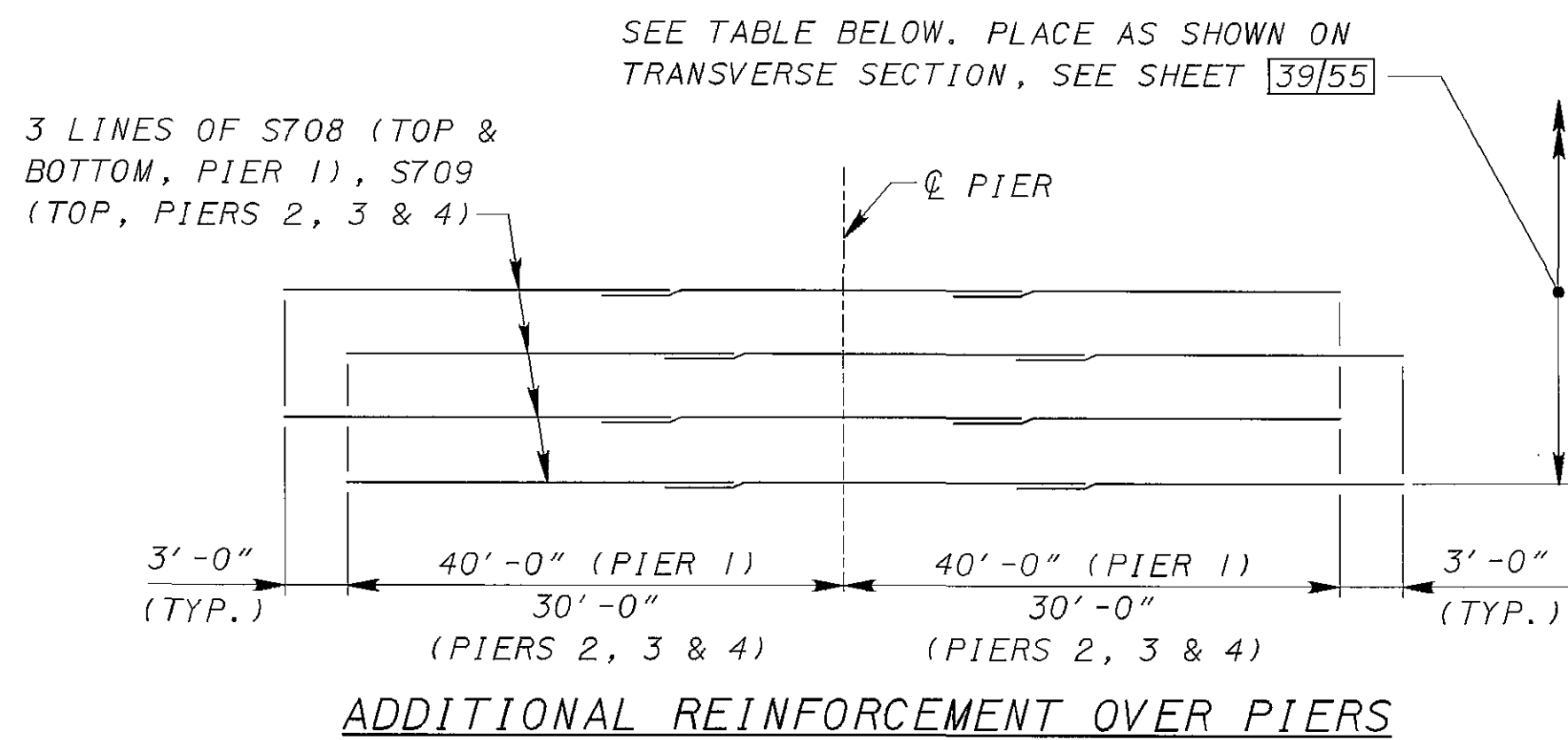
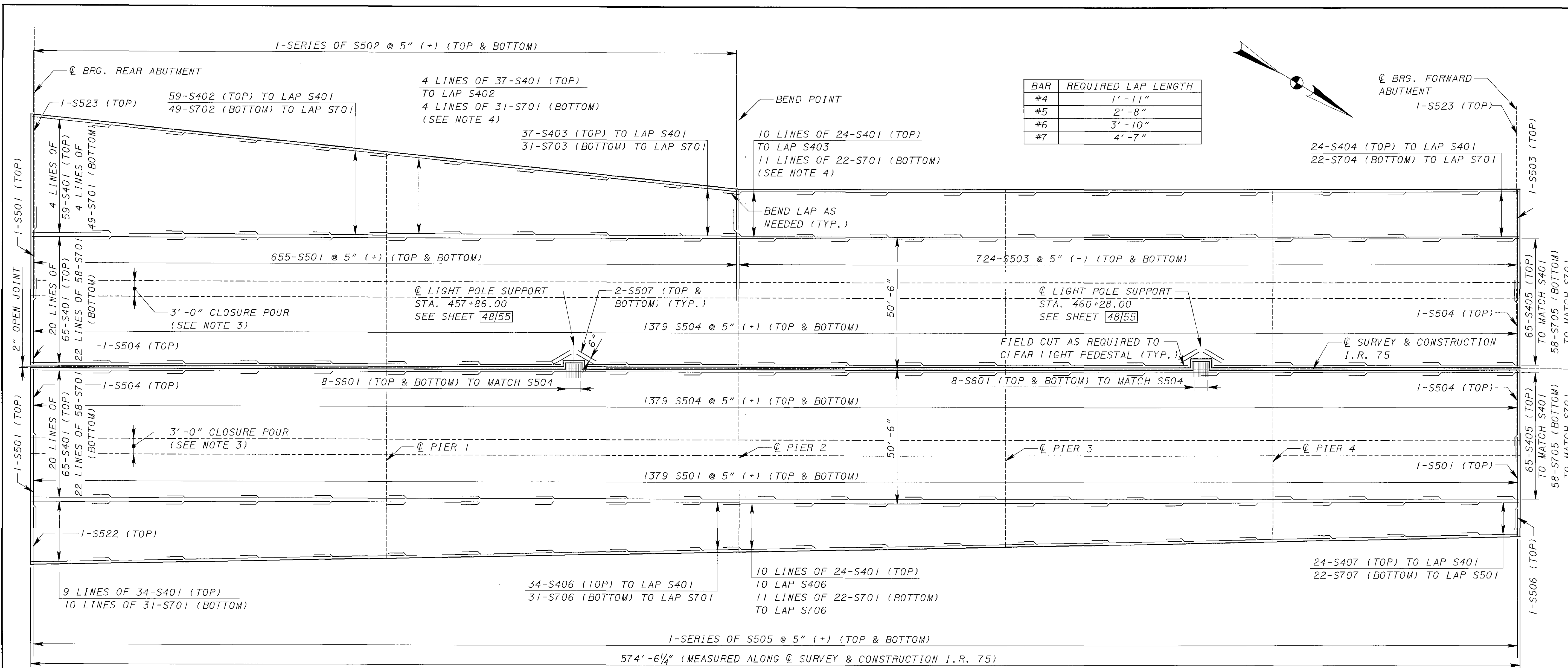
THE 2'-0" WIDTH OF THE PEDESTAL SHALL BE MEASURED PARALLEL TO THE CENTERLINE OF BEARING. THE FA901 AND FA602 BARS SHALL BE PLACED PARALLEL TO THE CENTERLINE OF BEARING. THE FA601 AND FA603 BARS SHALL BE PLACED PARALLEL TO THE BEAMS OR GIRDERS.



THE LOCATION OF THE MAIN REINFORCEMENT IN THE BEAM SEAT MAY BE ADJUSTED HORIZONTALLY ±1" TO ACCOMMODATE THE FA901 BARS.
 * - THE SURFACE OF THE BEAM SEAT IN THIS AREA SHALL BE FINISHED WITH A SERRATED TROWEL. THE SERRATIONS SHALL BE 1/4" DEEP MINIMUM.

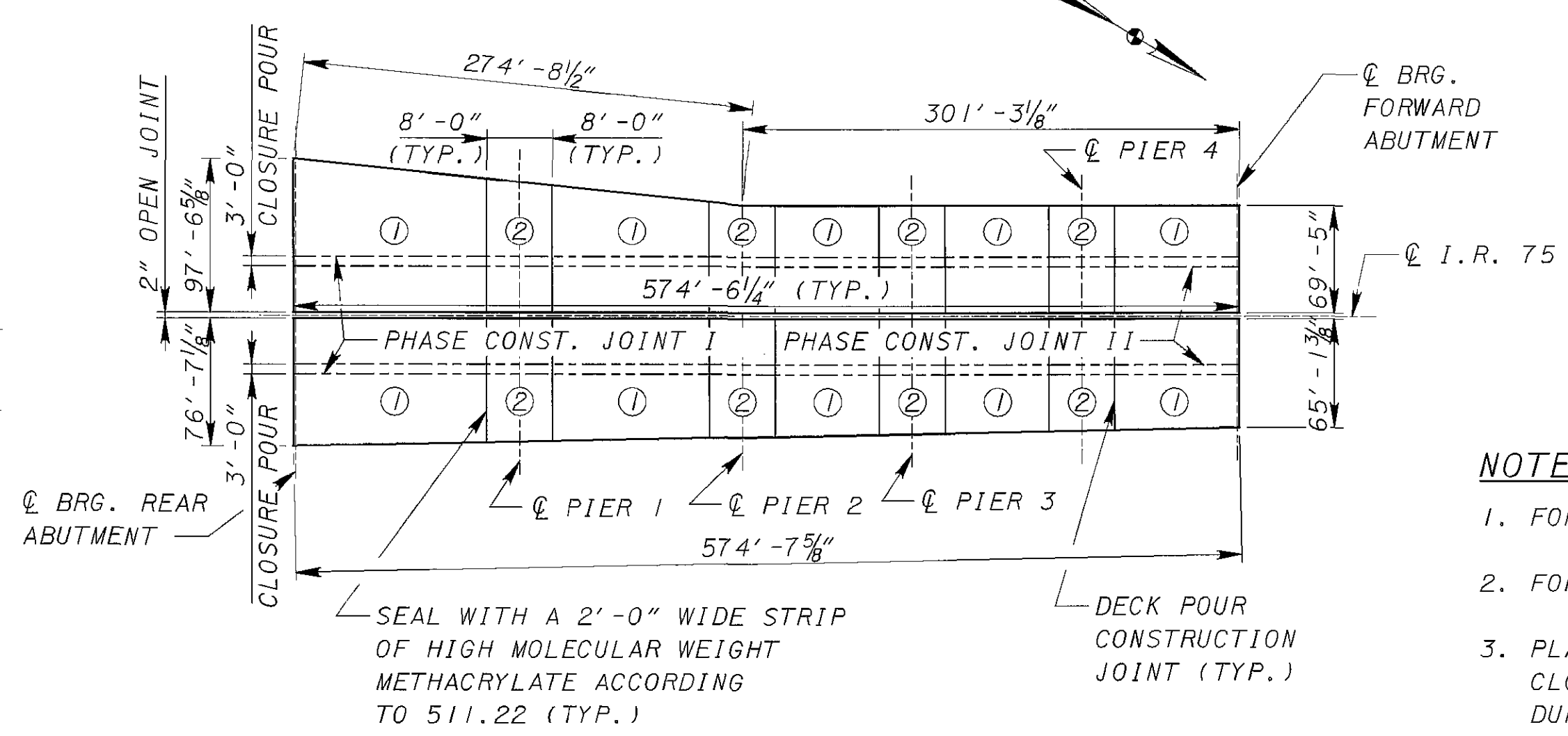
- NOTES:**
- FOR PIER DIAPHRAGM ELEVATIONS, SHEET 36/55.
 - FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
 - FOR DETAILS NOT SHOWN, REFER TO ODOT STANDARD DRAWING PISD-1-99.
 - DIAPHRAGMS AT CLOSURE POUR TO BE CAST DURING PHASE II CONSTRUCTION.
 - FOR DOWEL BAR DETAILS, SEE BEARING DETAILS SHEET 49/55.
 - FOR PIER DETAILS SEE SHEETS 22-25/55.

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NO. OF LINES OF ADDITIONAL REINFORCEMENT			
PIER	BRIDGE	TOP LAYER	BOTTOM LAYER
1	LEFT	123	94
	RIGHT	98	78
2	LEFT	101	-
	RIGHT	98	-
3	LEFT	88	-
	RIGHT	88	-
4	LEFT	88	-
	RIGHT	88	-

SLAB PLAN



DECK POURING SEQUENCE *

- ① POUR 1 - DECK POUR TO BE COMPLETED FIRST.
 - ② POUR 2 - DECK AND PIER DIAPHRAGM POUR TO BE PLACED MONOLITHICALLY AFTER ADJACENT DECK POURS.
- * END DIAPHRAGMS TO BE PLACED BEFORE ADJACENT DECK POURS.

NOTES:

1. FOR PARAPET DETAILS, SEE SHEETS 47/55 AND 48/55.
2. FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
3. PLACE LONGITUDINAL REINFORCING STEEL ON EITHER SIDE OF CLOSURE POUR TO PROVIDE 2" CLEARANCE TO REINFORCEMENT DURING ALL CONSTRUCTION PHASES.
4. AT PIERS 1 & 2 IN THE TAPERED SPANS, LAP A LIMITED NUMBER OF BOTTOM LONGITUDINAL REINFORCING BARS. THE NUMBER OF LAPS SHALL MATCH THE NUMBER OF BARS IN ADJACENT SPAN.
5. FOR ADDITIONAL DECK POUR NOTES, SEE ODOT STANDARD DRAWING PSID-1-99.

SLAB PLAN
 (PRESTRESSED CONCRETE OPTION)
 BRIDGE NO. MOT-75-1523
 I.R. 75 OVER THE GREAT MIAMI RIVER

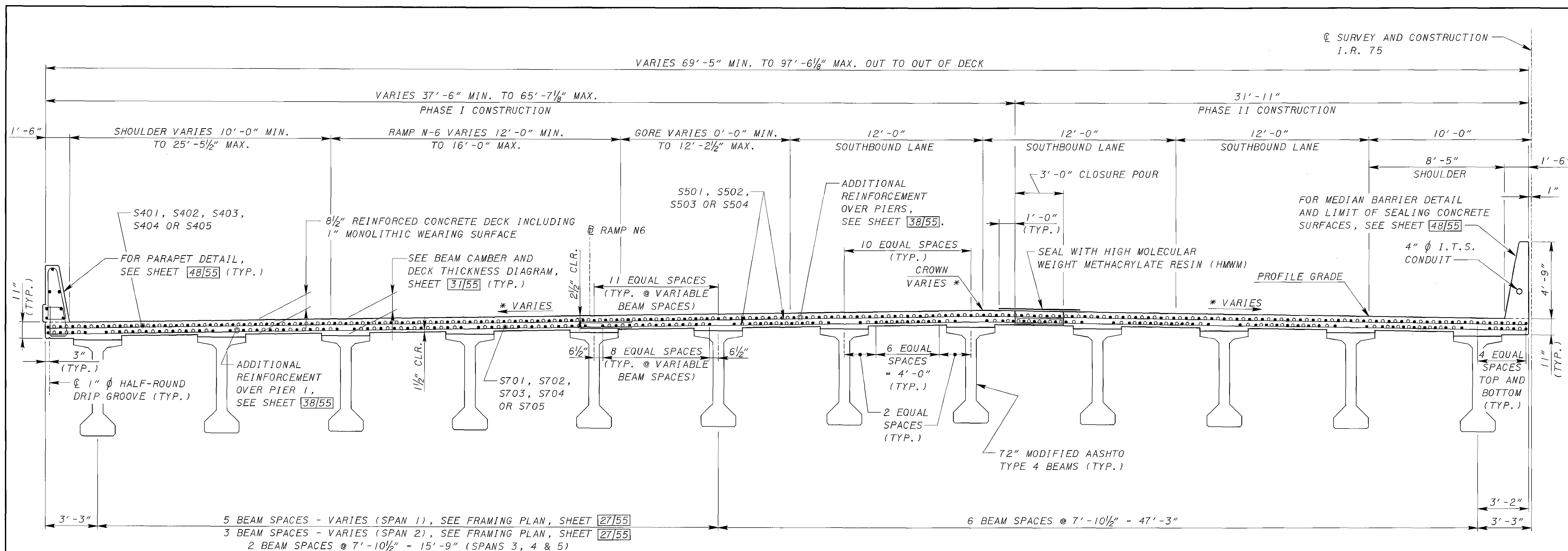
DESIGNED	BTA	CHECKED	JDH
DRAWN	MLR	REVIEWED	
DATE	05/27/05	STRUCTURE FILE NUMBER	5708710
DESIGN AGENCY			

MOT-75-14.60
PID 23828

38 / 55

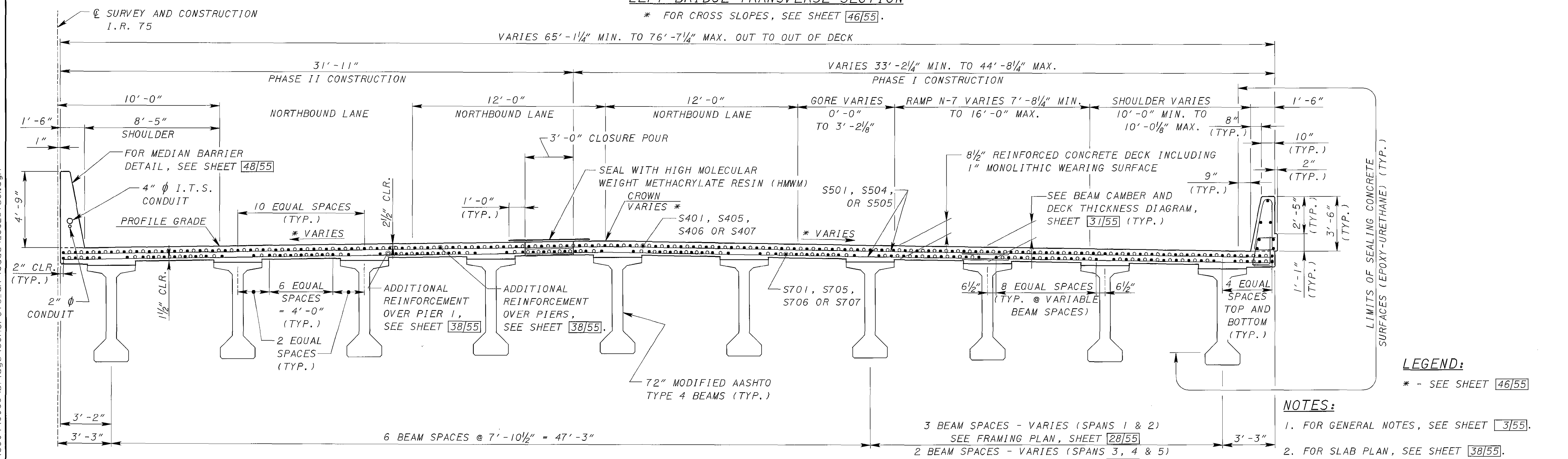
273 / 314

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:38 PM
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LEFT BRIDGE TRANSVERSE SECTION

* FOR CROSS SLOPES, SEE SHEET 46/55.



RIGHT BRIDGE TRANSVERSE SECTION

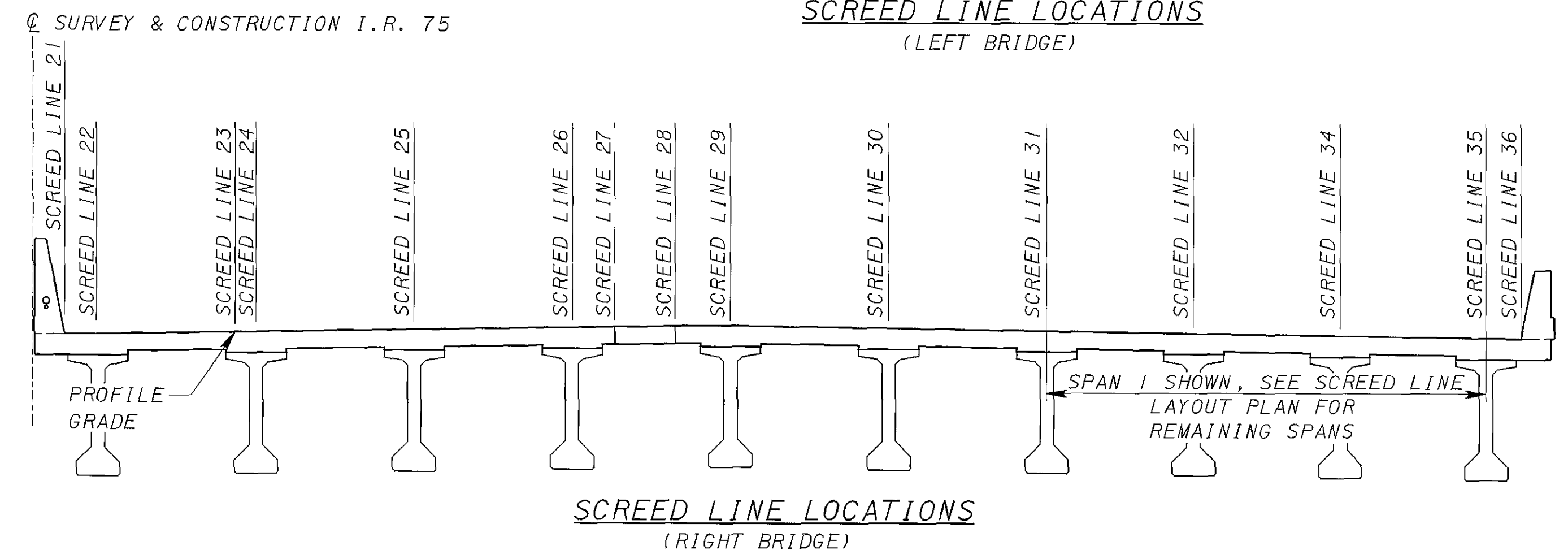
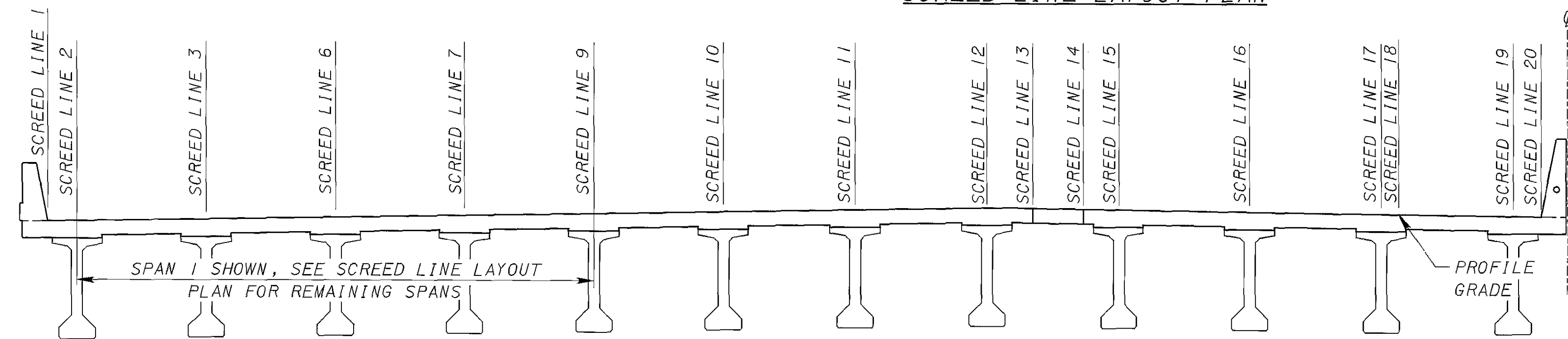
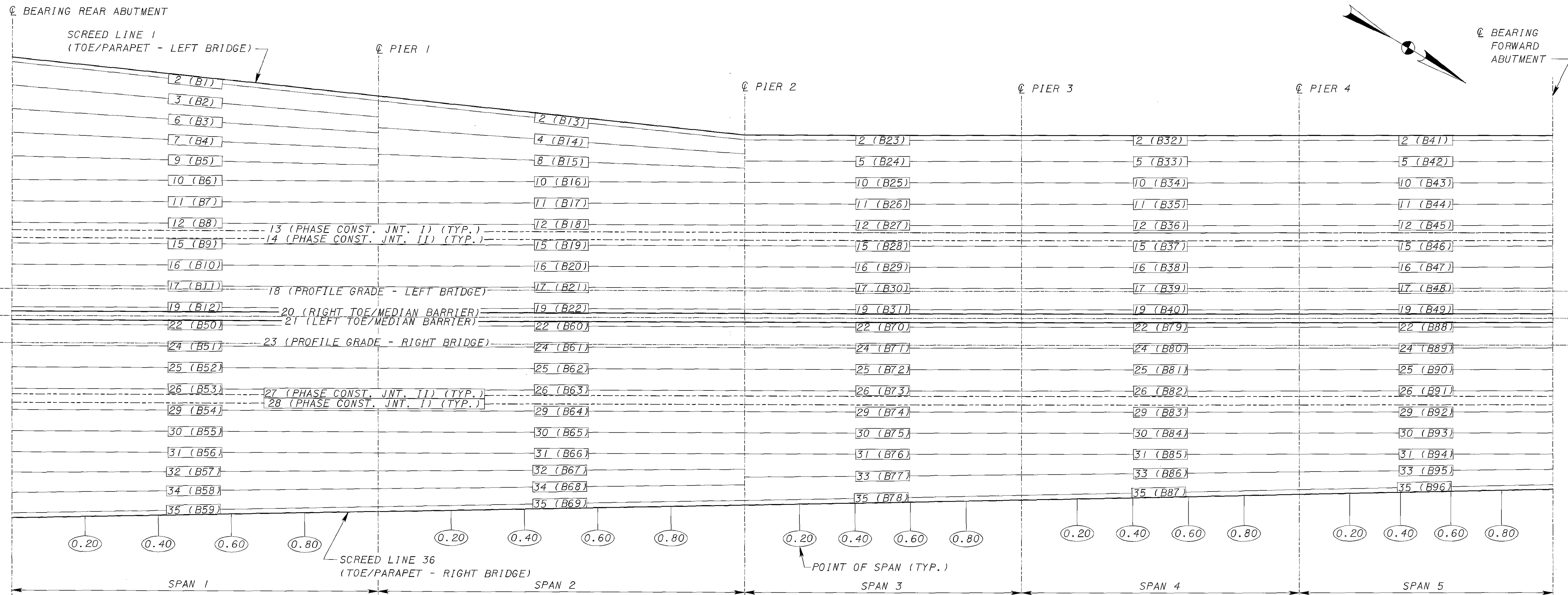
* FOR CROSS SLOPES, SEE SHEET 46/55.

LEGEND:
 * - SEE SHEET 46/55

- NOTES:**
1. FOR GENERAL NOTES, SEE SHEET 3/55.
 2. FOR SLAB PLAN, SEE SHEET 38/55.
 3. COST OF THE 4" Ø I.T.S. CONDUITS AND 2" Ø CONDUIT SHALL BE INCLUDED WITH ITEM 898, QC/QA CONCRETE, CLASS QSQC, SUPERSTRUCTURE (PARAPET)

	DESIGN AGENCY	DATE	REVIEWED	DATE
	TRAN SYSTEMS	05/27/05	NFF	05/27/05
	DESIGNED	CHECKED	DRAWN	REVISION
	BTA	JDH	CAG	5708710
TRANSVERSE SECTION (PRESTRESSED CONCRETE OPTION)				
BRIDGE NO. MOT-75-1523 I.R. 75 OVER THE GREAT MIAMI RIVER				
MOT-75-14.60 PID 23828				
39/55				
274 314				

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:29 PM
 Filename: g:\coo4\0066\brldge\concrete.dwg\cadd\23828sdl3.dgn



NOTES:
 SCREED ELEVATIONS SHOWN ARE FOR THE DECK SLAB SURFACE PRIOR TO CONCRETE PLACEMENT. ALLOWANCE HAS BEEN MADE FOR ANTICIPATED CALCULATED DEAD LOAD DEFLECTIONS.

DESIGN AGENCY TranSystems <small>5500 BUCKINGHAM SUITE #300 CLEVELAND, OHIO 44135-8800</small>	DATE 05/27/05
	REVIEWED NFF STRUCTURE FILE NUMBER 5708710
DRAWN CAG	CHECKED JDH
BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION) I.R. 75 OVER THE GREAT MIAMI RIVER	
MOT-75-14.60 PID 23828	
40 / 55 275 / 314	

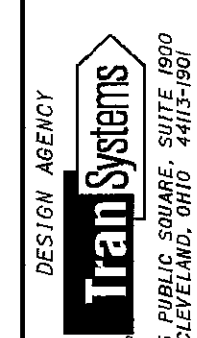
SCREED ELEVATIONS - LEFT BRIDGE SPANS 1 AND 2

LOCATION		℄ BEARING REAR ABUT.	0.2	0.4	0.5	0.6	0.8	℄ PIER 1	0.2	0.4	0.5	0.6	0.8	℄ PIER 2
SCREED LINE	STATION	455+77.61	456+04.81	456+32.01	456+45.61	456+59.21	456+86.41	457+13.61	457+40.81	457+68.01	457+81.61	457+95.21	458+22.41	458+49.61
LINE 1 TOE OF PARAPET (LEFT)	FINAL DECK ELEVATION	764.89	765.55	766.26	766.64	767.03	767.87	768.91	769.84	770.72	771.11	771.47	771.87	772.11
	SCREED ELEVATION	764.89	765.66	766.43	766.82	767.20	767.98	768.91	769.94	770.89	771.28	771.64	771.97	772.11
LINE 2	FINAL DECK ELEVATION	765.01	765.67	766.38	766.75	767.14	767.98	769.00	769.91	770.77	771.15	771.50	771.89	772.14
	SCREED ELEVATION	765.01	765.78	766.55	766.93	767.31	768.09	769.00	770.01	770.94	771.32	771.67	771.99	772.14
LINE 3	FINAL DECK ELEVATION	765.58	766.19	766.86	767.21	767.58	767.38	769.30	-	-	-	-	-	-
	SCREED ELEVATION	765.58	766.30	767.03	767.39	767.75	767.49	769.30	-	-	-	-	-	-
LINE 4	FINAL DECK ELEVATION	-	-	-	-	-	-	769.50	770.28	771.01	771.34	771.63	771.99	772.22
	SCREED ELEVATION	-	-	-	-	-	-	769.50	770.38	771.18	771.51	771.80	772.09	772.22
LINE 5	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	772.26
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	772.26
LINE 6	FINAL DECK ELEVATION	766.14	766.71	767.34	767.68	768.02	768.76	769.60	-	-	-	-	-	-
	SCREED ELEVATION	766.14	766.82	767.51	767.86	768.19	768.87	769.60	-	-	-	-	-	-
LINE 7	FINAL DECK ELEVATION	766.70	767.23	767.82	768.14	768.46	769.15	769.90	-	-	-	-	-	-
	SCREED ELEVATION	766.70	767.34	767.99	768.32	768.63	769.26	769.90	-	-	-	-	-	-
LINE 8	FINAL DECK ELEVATION	-	-	-	-	-	-	770.00	770.64	771.25	771.52	771.76	772.09	772.30
	SCREED ELEVATION	-	-	-	-	-	-	770.00	770.74	771.42	771.69	771.93	772.19	772.30
LINE 9	FINAL DECK ELEVATION	767.27	767.76	768.30	768.60	768.91	769.54	770.20	-	-	-	-	-	-
	SCREED ELEVATION	767.27	767.87	768.47	768.78	769.08	769.65	770.20	-	-	-	-	-	-
LINE 10	FINAL DECK ELEVATION	767.58	768.21	768.79	769.06	769.35	769.92	770.49	771.01	771.49	771.70	771.89	772.19	772.39
	SCREED ELEVATION	767.58	768.32	768.96	769.24	769.52	770.03	770.49	771.11	771.66	771.87	772.06	772.29	772.39
LINE 11	FINAL DECK ELEVATION	767.82	768.51	769.14	769.44	769.73	770.27	770.79	771.26	771.68	771.86	772.02	772.32	772.51
	SCREED ELEVATION	767.82	768.62	769.31	769.62	769.90	770.38	770.79	771.36	771.85	772.03	772.19	772.42	772.51
LINE 12	FINAL DECK ELEVATION	768.04	768.73	769.36	769.67	769.95	770.48	770.97	771.42	771.81	771.99	772.15	772.44	772.64
	SCREED ELEVATION	768.04	768.84	769.53	769.85	770.12	770.59	770.97	771.52	771.98	772.16	772.32	772.54	772.64
LINE 13 (PHASE CONST. JOINT I)	FINAL DECK ELEVATION	768.10	768.77	769.40	769.70	769.98	770.51	770.99	771.42	771.81	771.98	772.14	772.43	772.62
	SCREED ELEVATION	768.10	768.88	769.57	769.88	770.15	770.62	770.99	771.52	771.98	772.15	772.31	772.53	772.62
LINE 14 (PHASE CONST. JOINT II)	FINAL DECK ELEVATION	768.14	768.81	769.43	769.72	770.00	770.52	770.99	771.41	771.78	771.95	772.11	772.38	772.57
	SCREED ELEVATION	768.14	768.92	769.60	769.90	770.17	770.63	770.99	771.51	771.95	772.12	772.28	772.48	772.57
LINE 15	FINAL DECK ELEVATION	768.18	768.84	769.45	769.73	770.01	770.52	770.99	771.40	771.77	771.93	772.08	772.35	772.54
	SCREED ELEVATION	768.18	768.95	769.62	769.91	770.18	770.63	770.99	771.50	771.94	772.10	772.25	772.45	772.54
LINE 16	FINAL DECK ELEVATION	768.30	768.94	769.52	769.79	770.06	770.54	770.98	771.37	771.70	771.86	772.00	772.24	772.41
	SCREED ELEVATION	768.30	769.05	769.69	769.97	770.23	770.65	770.98	771.47	771.87	772.03	772.17	772.34	772.41
LINE 17	FINAL DECK ELEVATION	768.43	769.04	769.59	769.85	770.10	770.56	770.97	771.33	771.64	771.78	771.91	772.12	772.29
	SCREED ELEVATION	768.43	769.15	769.76	770.03	770.27	770.67	770.97	771.43	771.81	771.95	772.08	772.22	772.29
LINE 18 PROFILE GRADE (SOUTHBOUND)	FINAL DECK ELEVATION	768.45	769.05	769.60	769.86	770.11	770.56	770.97	771.33	771.64	771.77	771.89	772.11	772.27
	SCREED ELEVATION	768.45	769.16	769.77	770.04	770.28	770.67	770.97	771.43	771.81	771.94	772.06	772.21	772.27
LINE 19	FINAL DECK ELEVATION	768.34	768.94	769.50	769.75	770.00	770.46	770.86	771.22	771.53	771.66	771.79	772.00	772.16
	SCREED ELEVATION	768.34	769.05	769.67	769.93	770.17	770.57	770.86	771.32	771.70	771.83	771.96	772.10	772.16
LINE 20 TOE OF MEDIAN BARRIER (RIGHT)	FINAL DECK ELEVATION	768.31	768.92	769.47	769.73	769.97	770.43	770.83	771.19	771.50	771.63	771.76	771.97	772.13
	SCREED ELEVATION	768.31	769.03	769.64	769.91	770.14	770.54	770.83	771.29	771.67	771.80	771.93	772.07	772.13

NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET 40/55.
- FOR SCREED ELEVATIONS, SPANS 3, 4 AND 5, SEE SHEETS 42/55 AND 43/55.
- FOR TABLE OF ANTICIPATED DEAD LOAD DEFLECTIONS, SEE SHEET 43/55.

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:29 PM
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DESIGN AGENCY
DATE 5/31/05
REVIEWED NFF
STRUCTURE FILE NUMBER 5708710

DRAWN BTA
DESIGNED BTA
CHECKED JDH

SCREED ELEVATIONS - LEFT BRIDGE SPANS 1 AND 2
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 IR-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

41/55

276
314

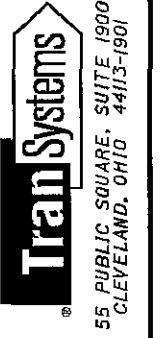
SCREED ELEVATIONS - LEFT BRIDGE SPANS 3 AND 4

LOCATION		☉ PIER 2	0.2	0.4	0.5	0.6	0.8	☉ PIER 3	0.2	0.4	0.5	0.6	0.8	☉ PIER 4
SCREED LINE	STATION	458+49.61	458+70.21	458+90.81	459+01.11	459+11.41	459+32.01	459+52.61	459+73.21	459+93.81	460+04.11	460+14.41	460+35.01	460+55.61
LINE 1 TOE OF PARAPET (LEFT)	FINAL DECK ELEVATION	772.11	772.20	772.26	772.28	772.29	772.30	772.28	772.23	772.15	772.10	772.04	771.91	771.74
	SCREED ELEVATION	772.11	772.23	772.31	772.34	772.34	772.33	772.28	772.26	772.20	772.16	772.09	771.94	771.74
LINE 2	FINAL DECK ELEVATION	772.14	772.23	772.29	772.31	772.32	772.33	772.30	772.25	772.18	772.13	772.07	771.94	771.77
	SCREED ELEVATION	772.14	772.26	772.34	772.37	772.37	772.36	772.30	772.28	772.23	772.19	772.12	771.97	771.77
LINE 3	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
LINE 4	FINAL DECK ELEVATION	772.22	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	772.22	-	-	-	-	-	-	-	-	-	-	-	-
LINE 5	FINAL DECK ELEVATION	772.26	772.35	772.41	772.43	772.45	772.45	772.43	772.38	772.30	772.25	772.20	772.06	771.90
	SCREED ELEVATION	772.26	772.38	772.46	772.49	772.50	772.48	772.43	772.41	772.35	772.31	772.25	772.09	771.90
LINE 6	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
LINE 7	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
LINE 8	FINAL DECK ELEVATION	772.30	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	772.30	-	-	-	-	-	-	-	-	-	-	-	-
LINE 9	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	-
LINE 10	FINAL DECK ELEVATION	772.39	772.48	772.54	772.56	772.57	772.58	772.66	772.51	772.43	772.38	772.32	772.19	772.02
	SCREED ELEVATION	772.39	772.51	772.59	772.62	772.62	772.61	772.66	772.54	772.48	772.44	772.37	772.22	772.02
LINE 11	FINAL DECK ELEVATION	772.51	772.60	772.67	772.69	772.70	772.70	772.68	772.63	772.55	772.50	772.45	772.31	772.15
	SCREED ELEVATION	772.51	772.63	772.72	772.75	772.75	772.73	772.68	772.66	772.60	772.56	772.50	772.34	772.15
LINE 12	FINAL DECK ELEVATION	772.64	772.73	772.79	772.81	772.82	772.83	772.81	772.76	772.68	772.63	772.57	772.44	772.28
	SCREED ELEVATION	772.64	772.76	772.84	772.87	772.87	772.86	772.81	772.79	772.73	772.69	772.62	772.47	772.28
LINE 13 (PHASE CONST. JOINT 1)	FINAL DECK ELEVATION	772.62	772.71	772.77	772.79	772.80	772.81	772.79	772.74	772.66	772.61	772.55	772.42	772.26
	SCREED ELEVATION	772.62	772.74	772.82	772.85	772.85	772.84	772.79	772.77	772.71	772.67	772.60	772.45	772.26
LINE 14 (PHASE CONST. JOINT 11)	FINAL DECK ELEVATION	772.57	772.66	772.72	772.74	772.76	772.76	772.74	772.69	772.61	772.56	772.51	772.37	772.21
	SCREED ELEVATION	772.57	772.69	772.77	772.80	772.81	772.79	772.74	772.72	772.66	772.62	772.56	772.40	772.21
LINE 15	FINAL DECK ELEVATION	772.54	772.63	772.69	772.71	772.72	772.73	772.71	772.66	772.58	772.53	772.47	772.34	772.17
	SCREED ELEVATION	772.54	772.66	772.74	772.77	772.77	772.76	772.71	772.69	772.63	772.59	772.52	772.37	772.17
LINE 16	FINAL DECK ELEVATION	772.41	772.50	772.56	772.58	772.60	772.60	772.58	772.53	772.45	772.40	772.35	772.21	772.05
	SCREED ELEVATION	772.41	772.53	772.61	772.64	772.65	772.63	772.58	772.56	772.50	772.46	772.40	772.24	772.05
LINE 17	FINAL DECK ELEVATION	772.29	772.38	772.44	772.46	772.47	772.48	772.45	772.40	772.33	772.28	772.22	772.09	771.92
	SCREED ELEVATION	772.29	772.41	772.49	772.52	772.52	772.51	772.45	772.43	772.38	772.34	772.27	772.12	771.92
LINE 18 PROFILE GRADE (SOUTHBOUND)	FINAL DECK ELEVATION	772.27	772.36	772.42	772.44	772.45	772.46	772.44	772.39	772.31	772.26	772.20	772.07	771.90
	SCREED ELEVATION	772.27	772.39	772.47	772.50	772.50	772.49	772.44	772.42	772.36	772.32	772.25	772.10	771.90
LINE 19	FINAL DECK ELEVATION	772.16	772.25	772.31	772.33	772.34	772.35	772.33	772.28	772.20	772.15	772.09	771.96	771.80
	SCREED ELEVATION	772.16	772.28	772.36	772.39	772.39	772.38	772.33	772.31	772.25	772.21	772.14	771.99	771.80
LINE 20 TOE OF MEDIAN BARRIER (RIGHT)	FINAL DECK ELEVATION	772.13	772.22	772.28	772.31	772.32	772.32	772.30	772.25	772.17	772.13	772.07	771.93	771.77
	SCREED ELEVATION	772.13	772.25	772.33	772.37	772.37	772.35	772.30	772.28	772.22	772.19	772.12	771.96	771.77

NOTES:

1. FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET 40/55.
2. FOR SCREED ELEVATIONS, SPANS 1, 2 AND 5, SEE SHEETS 41/55 AND 43/55.
3. FOR TABLE OF ANTICIPATED DEAD LOAD DEFLECTIONS, SEE SHEET 43/55.

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:30 PM
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DESIGN AGENCY
 DATE 5/31/05
 REVIEWED NFF
 STRUCTURE FILE NUMBER 57087.10

DRAWN BTA
 CHECKED J.D.H.

SCREED ELEVATIONS - LEFT BRIDGE SPANS 3 AND 4
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 I.R-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:30 PM
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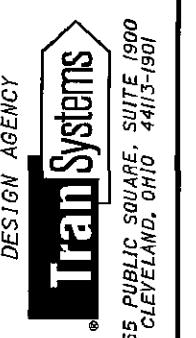
SCREED ELEVATIONS - LEFT BRIDGE SPAN 5

LOCATION		☉ PIER 4	0.2	0.4	0.5	0.6	0.8	☉ BEARING FWD. ABUT.
SCREED LINE	STATION	460+55.61	460+74.41	460+93.21	461+02.61	461+12.01	461+30.81	461+49.61
LINE 1 TOE OF PARAPET (LEFT)	FINAL DECK ELEVATION	771.74	771.57	771.38	771.27	771.16	770.91	770.65
	SCREED ELEVATION	771.74	771.59	771.42	771.31	771.20	770.93	770.65
LINE 2	FINAL DECK ELEVATION	771.77	771.60	771.40	771.29	771.18	770.94	770.68
	SCREED ELEVATION	771.77	771.62	771.44	771.33	771.22	770.96	770.68
LINE 3	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 4	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 5	FINAL DECK ELEVATION	771.90	771.73	771.53	771.42	771.31	771.07	770.80
	SCREED ELEVATION	771.90	771.75	771.57	771.46	771.35	771.09	770.80
LINE 6	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 7	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 8	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 9	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 10	FINAL DECK ELEVATION	772.02	771.85	771.66	771.55	771.44	771.19	770.93
	SCREED ELEVATION	772.02	771.87	771.70	771.59	771.48	771.21	770.93
LINE 11	FINAL DECK ELEVATION	772.15	771.98	771.78	771.67	771.56	771.32	771.05
	SCREED ELEVATION	772.15	772.00	771.82	771.71	771.60	771.34	771.05
LINE 12	FINAL DECK ELEVATION	772.28	772.10	771.91	771.80	771.69	771.45	771.18
	SCREED ELEVATION	772.28	772.12	771.95	771.84	771.73	771.47	771.18
LINE 13 (PHASE CONST. JOINT I)	FINAL DECK ELEVATION	772.26	772.08	771.89	771.78	771.67	771.43	771.16
	SCREED ELEVATION	772.26	772.10	771.93	771.82	771.71	771.45	771.16
LINE 14 (PHASE CONST. JOINT II)	FINAL DECK ELEVATION	772.21	772.04	771.84	771.73	771.62	771.38	771.11
	SCREED ELEVATION	772.21	772.06	771.88	771.77	771.66	771.40	771.11
LINE 15	FINAL DECK ELEVATION	772.17	772.00	771.81	771.70	771.59	771.34	771.08
	SCREED ELEVATION	772.17	772.02	771.85	771.74	771.63	771.36	771.08
LINE 16	FINAL DECK ELEVATION	772.05	771.88	771.68	771.57	771.46	771.22	770.95
	SCREED ELEVATION	772.05	771.90	771.72	771.61	771.50	771.24	770.95
LINE 17	FINAL DECK ELEVATION	771.92	771.75	771.55	771.45	771.33	771.09	770.83
	SCREED ELEVATION	771.92	771.77	771.59	771.49	771.37	771.11	770.83
LINE 18 PROFILE GRADE (SOUTHBOUND)	FINAL DECK ELEVATION	771.90	771.73	771.54	771.43	771.32	771.07	770.81
	SCREED ELEVATION	771.90	771.75	771.58	771.47	771.36	771.09	770.81
LINE 19	FINAL DECK ELEVATION	771.80	771.62	771.43	771.32	771.21	770.97	770.70
	SCREED ELEVATION	771.80	771.64	771.47	771.36	771.25	770.99	770.70
LINE 20 TOE OF MEDIAN BARRIER (RIGHT)	FINAL DECK ELEVATION	771.77	771.60	771.40	771.29	771.18	770.94	770.67
	SCREED ELEVATION	771.77	771.62	771.44	771.33	771.22	770.96	770.67

	0.0	0.2	0.4	0.5	0.6	0.8	1.0
SPAN 1 (☉ BEARING REAR ABUT. TO ☉ PIER 1)	0.000	1.268	2.062	2.167	2.062	1.268	0.000
SPAN 2 (☉ PIER 1 TO ☉ PIER 2)	0.000	1.221	1.989	2.091	1.989	1.221	0.000
SPAN 3 (☉ PIER 2 TO ☉ PIER 3)	0.000	0.390	0.639	0.672	0.639	0.390	0.000
SPAN 4 (☉ PIER 3 TO ☉ PIER 4)	0.000	0.390	0.639	0.672	0.639	0.390	0.000
SPAN 5 (☉ PIER 4 TO ☉ BEARING FWD. ABUT.)	0.000	0.282	0.463	0.487	0.463	0.282	0.000

NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET 40/55.
- FOR SCREED ELEVATIONS, SPANS 1, 2, 3 AND 4, SEE SHEETS 41/55 AND 42/55.



DESIGNED BY: BTA
 CHECKED BY: JDH
 DRAWN BY: BTA
 REVISED BY:
 REVIEWED BY: NFF
 DATE: 5/31/05
 STRUCTURE FILE NUMBER: 5708710

SCREED ELEVATIONS - LEFT BRIDGE SPAN 5
 (PRESTRESSED CONCRETE OPTION)
 BRIDGE NO. MOT-75-1523
 I.R-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

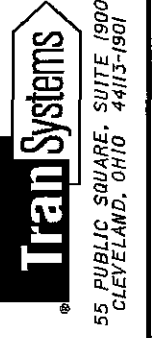
SCREED ELEVATIONS - RIGHT BRIDGE SPANS 1 AND 2

LOCATION		☉ BEARING REAR ABUT.	0.2	0.4	0.5	0.6	0.8	☉ PIER 1	0.2	0.4	0.5	0.6	0.8	☉ PIER 2
SCREED LINE	STATION	455+77.61	456+04.81	456+32.01	456+45.61	456+59.21	456+86.41	457+13.61	457+40.81	457+68.01	457+81.61	457+95.21	458+22.41	458+49.61
LINE 21 TOE OF MEDIAN BARRIER (LEFT)	FINAL DECK ELEVATION	768.31	768.92	769.47	769.73	769.97	770.43	770.83	771.19	771.50	771.63	771.76	771.97	772.13
	SCREED ELEVATION	768.31	769.03	769.64	769.91	770.14	770.54	770.83	771.29	771.67	771.80	771.93	772.07	772.13
LINE 22	FINAL DECK ELEVATION	768.34	768.94	769.50	769.75	770.00	770.46	770.86	771.22	771.53	771.66	771.79	772.00	772.16
	SCREED ELEVATION	768.34	769.05	769.67	769.93	770.17	770.57	770.86	771.32	771.70	771.83	771.96	772.10	772.16
LINE 23 PROFILE GRADE (NORTHBOUND)	FINAL DECK ELEVATION	768.45	769.05	769.60	769.86	770.11	770.56	770.97	771.33	771.64	771.77	771.89	772.11	772.27
	SCREED ELEVATION	768.45	769.16	769.77	770.04	770.28	770.67	770.97	771.43	771.81	771.94	772.06	772.21	772.27
LINE 24	FINAL DECK ELEVATION	768.47	769.07	769.62	769.88	770.13	770.58	770.99	771.34	771.65	771.79	771.91	772.12	772.29
	SCREED ELEVATION	768.47	769.18	769.79	770.06	770.30	770.69	770.99	771.44	771.82	771.96	772.08	772.22	772.29
LINE 25	FINAL DECK ELEVATION	768.59	769.20	769.75	770.01	770.25	770.71	771.11	771.47	771.78	771.92	772.04	772.25	772.41
	SCREED ELEVATION	768.59	769.31	769.92	770.19	770.42	770.82	771.11	771.57	771.95	772.09	772.21	772.35	772.41
LINE 26	FINAL DECK ELEVATION	768.72	769.32	769.87	770.13	770.38	770.83	771.24	771.60	771.91	772.04	772.16	772.38	772.54
	SCREED ELEVATION	768.72	769.43	770.04	770.31	770.55	770.94	771.24	771.70	772.08	772.21	772.33	772.48	772.54
LINE 27 (PHASE CONST. JOINT 11)	FINAL DECK ELEVATION	768.76	769.36	769.91	770.17	770.41	770.87	771.27	771.63	771.94	772.08	772.20	772.41	772.57
	SCREED ELEVATION	768.76	769.47	770.08	770.35	770.58	770.98	771.27	771.73	772.11	772.25	772.37	772.51	772.57
LINE 28 (PHASE CONST. JOINT 1)	FINAL DECK ELEVATION	768.80	769.40	769.96	770.21	770.46	770.92	771.32	771.68	771.99	772.13	772.25	772.46	772.62
	SCREED ELEVATION	768.80	769.51	770.13	770.39	770.63	771.03	771.32	771.78	772.16	772.30	772.42	772.56	772.62
LINE 29	FINAL DECK ELEVATION	768.85	769.44	770.00	770.25	770.50	770.95	771.35	771.71	772.01	772.15	772.27	772.48	772.64
	SCREED ELEVATION	768.85	769.55	770.17	770.43	770.67	771.06	771.35	771.81	772.18	772.32	772.44	772.58	772.64
LINE 30	FINAL DECK ELEVATION	768.97	769.54	770.07	770.31	770.54	770.97	771.35	771.67	771.95	772.07	772.18	772.36	772.51
	SCREED ELEVATION	768.97	769.65	770.24	770.49	770.71	771.08	771.35	771.77	772.12	772.24	772.35	772.46	772.51
LINE 31	FINAL DECK ELEVATION	768.96	769.52	770.03	770.27	770.49	770.91	771.27	771.59	771.85	771.97	772.07	772.24	772.39
	SCREED ELEVATION	768.96	769.63	770.20	770.45	770.66	771.02	771.27	771.69	772.02	772.14	772.24	772.34	772.39
LINE 32	FINAL DECK ELEVATION	768.84	769.40	769.91	770.15	770.38	770.79	771.16	771.48	771.75	771.87	771.97	772.15	772.29
	SCREED ELEVATION	768.84	769.51	770.08	770.33	770.55	770.90	771.16	771.58	771.92	772.04	772.14	772.25	772.29
LINE 33	FINAL DECK ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	772.25
	SCREED ELEVATION	-	-	-	-	-	-	-	-	-	-	-	-	772.25
LINE 34	FINAL DECK ELEVATION	768.71	769.28	769.79	770.04	770.26	770.68	771.06	771.38	771.65	771.77	771.88	772.05	772.20
	SCREED ELEVATION	768.71	769.39	769.96	770.22	770.43	770.79	771.06	771.48	771.82	771.94	772.05	772.15	772.20
LINE 35	FINAL DECK ELEVATION	768.59	769.16	769.68	769.92	770.15	770.57	770.95	771.27	771.55	771.67	771.78	771.95	772.11
	SCREED ELEVATION	768.59	769.27	769.85	770.10	770.32	770.68	770.95	771.37	771.72	771.84	771.95	772.05	772.11
LINE 36 TOE OF PARAPET (RIGHT)	FINAL DECK ELEVATION	768.56	769.13	769.65	769.89	770.12	770.55	770.92	771.24	771.52	771.64	771.75	771.93	772.08
	SCREED ELEVATION	768.56	769.24	769.82	770.07	770.29	770.66	770.92	771.34	771.69	771.81	771.92	772.03	772.08

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:31 PM
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NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET **40/55**.
- FOR SCREED ELEVATIONS, SPANS 3, 4 AND 5, SEE SHEETS **45/55** AND **46/55**.
- FOR TABLE OF ANTICIPATED DEAD LOAD DEFLECTIONS, SEE SHEET **43/55**.



DESIGN AGENCY
 TRAN SYSTEMS
 5100 N. W. 11th St.
 Fort Lauderdale, FL 33309

REVIEWED DATE 5/31/05
 NFF STRUCTURE FILE NUMBER 5708710

DESIGNED BY BTA
 CHECKED BY JDH

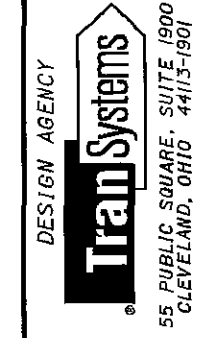
SCREED ELEVATIONS - RIGHT BRIDGE SPANS 1 AND 2
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 I/R-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:31PM
 Filename: g:\c004\0066\bridge\concrete_dit\c004\23828sdl8.dgn

SCREED ELEVATIONS - RIGHT BRIDGE SPANS 3 AND 4

LOCATION		☉ PIER 2	0.2	0.4	0.5	0.6	0.8	☉ PIER 3	0.2	0.4	0.5	0.6	0.8	☉ PIER 4
SCREED LINE	STATION	458+49.61	458+70.21	458+90.81	459+01.11	459+11.41	459+32.01	459+52.61	459+73.21	459+93.81	460+04.11	460+14.41	460+35.01	460+55.61
LINE 21 TOE OF MEDIAN BARRIER (LEFT)	FINAL DECK ELEVATION	772.13	772.22	772.28	772.31	772.32	772.32	772.30	772.25	772.17	772.13	772.07	771.93	771.77
	SCREED ELEVATION	772.13	772.25	772.33	772.37	772.37	772.35	772.30	772.28	772.22	772.19	772.12	771.96	771.77
LINE 22	FINAL DECK ELEVATION	772.16	772.25	772.31	772.33	772.34	772.35	772.33	772.28	772.20	772.15	772.09	771.96	771.80
	SCREED ELEVATION	772.16	772.28	772.36	772.39	772.39	772.38	772.33	772.31	772.25	772.21	772.14	771.99	771.80
LINE 23 PROFILE GRADE (NORTHBOUND)	FINAL DECK ELEVATION	772.27	772.36	772.42	772.44	772.45	772.46	772.44	772.39	772.31	772.26	772.20	772.07	771.90
	SCREED ELEVATION	772.27	772.39	772.47	772.50	772.50	772.49	772.44	772.42	772.36	772.32	772.25	772.10	771.90
LINE 24	FINAL DECK ELEVATION	772.29	772.38	772.44	772.46	772.47	772.48	772.45	772.40	772.33	772.28	772.22	772.09	771.92
	SCREED ELEVATION	772.29	772.41	772.49	772.52	772.52	772.51	772.45	772.43	772.38	772.34	772.27	772.12	771.92
LINE 25	FINAL DECK ELEVATION	772.41	772.50	772.56	772.58	772.60	772.60	772.58	772.53	772.45	772.40	772.35	772.21	772.05
	SCREED ELEVATION	772.41	772.53	772.61	772.64	772.65	772.63	772.58	772.56	772.50	772.46	772.40	772.24	772.05
LINE 26	FINAL DECK ELEVATION	772.54	772.63	772.69	772.71	772.72	772.73	772.71	772.66	772.58	772.53	772.47	772.34	772.17
	SCREED ELEVATION	772.54	772.66	772.74	772.77	772.77	772.76	772.71	772.69	772.63	772.59	772.52	772.37	772.17
LINE 27 (PHASE CONST. JOINT 11)	FINAL DECK ELEVATION	772.57	772.66	772.72	772.74	772.76	772.76	772.74	772.69	772.61	772.56	772.51	772.37	772.21
	SCREED ELEVATION	772.57	772.69	772.77	772.80	772.81	772.79	772.74	772.72	772.66	772.62	772.56	772.40	772.21
LINE 28 (PHASE CONST. JOINT 1)	FINAL DECK ELEVATION	772.62	772.71	772.77	772.79	772.80	772.81	772.79	772.74	772.66	772.61	772.55	772.42	772.26
	SCREED ELEVATION	772.62	772.74	772.82	772.85	772.85	772.84	772.79	772.77	772.71	772.67	772.60	772.45	772.26
LINE 29	FINAL DECK ELEVATION	772.64	772.73	772.79	772.81	772.82	772.83	772.81	772.76	772.68	772.63	772.57	772.44	772.28
	SCREED ELEVATION	772.64	772.76	772.84	772.87	772.87	772.86	772.81	772.79	772.73	772.69	772.62	772.47	772.28
LINE 30	FINAL DECK ELEVATION	772.51	772.60	772.67	772.69	772.70	772.70	772.68	772.63	772.55	772.50	772.45	772.31	772.15
	SCREED ELEVATION	772.51	772.63	772.72	772.75	772.75	772.73	772.68	772.66	772.60	772.56	772.50	772.34	772.15
LINE 31	FINAL DECK ELEVATION	772.39	772.48	772.54	772.56	772.57	772.58	772.56	772.51	772.43	772.38	772.32	772.19	772.02
	SCREED ELEVATION	772.39	772.51	772.59	772.62	772.62	772.61	772.56	772.54	772.48	772.44	772.37	772.22	772.02
LINE 32	FINAL DECK ELEVATION	772.29	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	772.29	-	-	-	-	-	-	-	-	-	-	-	-
LINE 33	FINAL DECK ELEVATION	772.25	772.34	772.41	772.43	772.44	772.45	772.43	772.39	772.31	772.26	772.21	772.08	771.92
	SCREED ELEVATION	772.25	772.37	772.46	772.49	772.49	772.48	772.43	772.42	772.36	772.32	772.26	772.11	771.92
LINE 34	FINAL DECK ELEVATION	772.20	-	-	-	-	-	-	-	-	-	-	-	-
	SCREED ELEVATION	772.20	-	-	-	-	-	-	-	-	-	-	-	-
LINE 35	FINAL DECK ELEVATION	772.11	772.20	772.27	772.30	772.31	772.33	772.31	772.27	772.19	772.15	772.09	771.97	771.81
	SCREED ELEVATION	772.11	772.23	772.32	772.36	772.36	772.36	772.31	772.30	772.24	772.21	772.14	772.00	771.81
LINE 36 TOE OF PARAPET (RIGHT)	FINAL DECK ELEVATION	772.08	772.18	772.24	772.27	772.29	772.30	772.28	772.24	772.17	772.12	772.07	771.94	771.78
	SCREED ELEVATION	772.08	772.21	772.29	772.33	772.34	772.33	772.28	772.27	772.22	772.18	772.12	771.97	771.78



DESIGN AGENCY
 DATE
 5/31/05
 STRUCTURE FILE NUMBER
 5708710

DESIGNED
 BTA
 CHECKED
 JDH

SCREED ELEVATIONS - RIGHT BRIDGE SPANS 3 AND 4
 (PRESTRESSED CONCRETE OPTION)

BRIDGE NO. MOT-75-1523
 IR-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

45/55

280
 314

NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET 40/55.
- FOR SCREED ELEVATIONS, SPANS 1, 2 AND 5, SEE SHEETS 44/55 AND 46/55.
- FOR TABLE OF ANTICIPATED DEAD LOAD DEFLECTIONS, SEE SHEET 43/55.

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SCREED ELEVATIONS - RIGHT BRIDGE SPAN 5

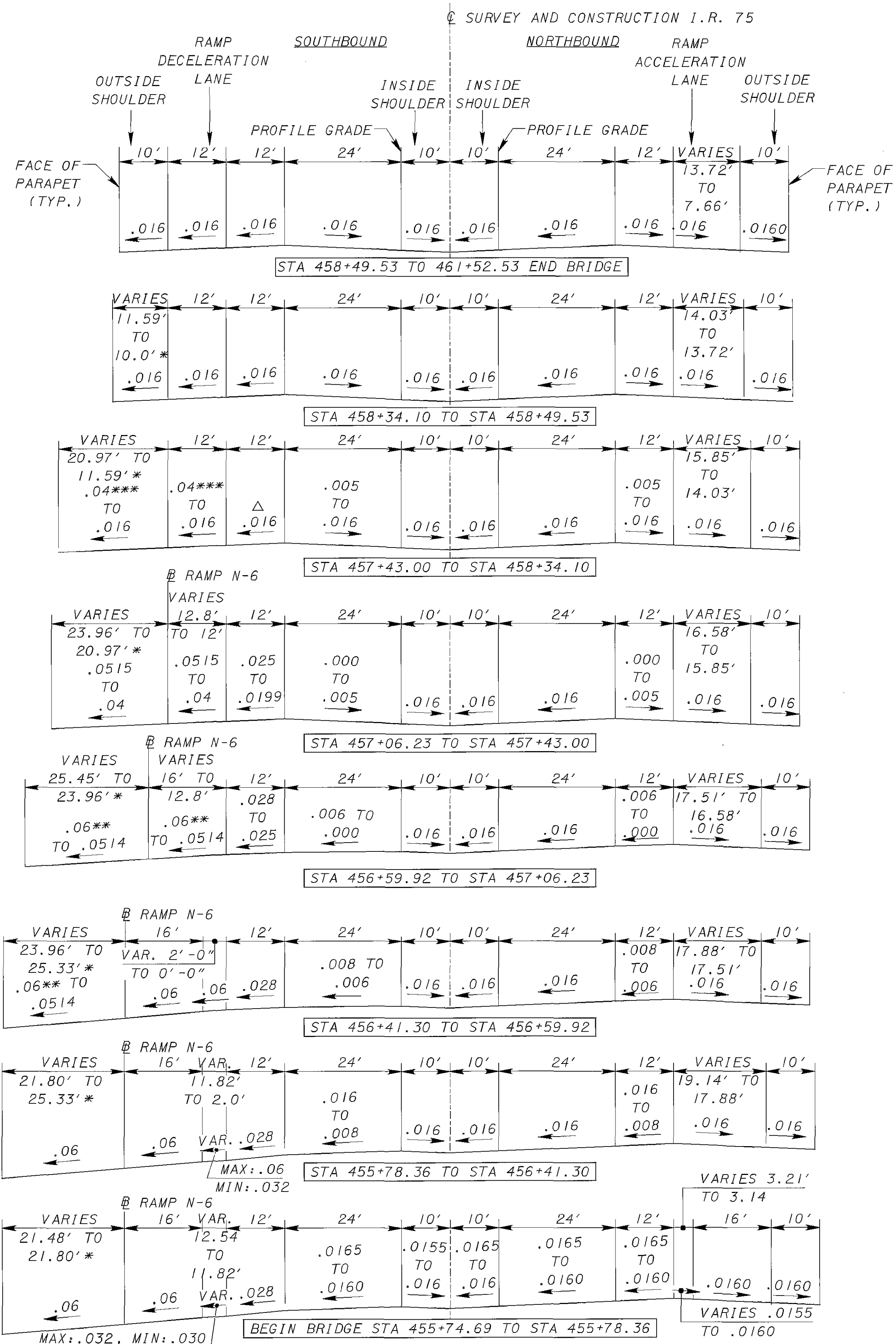
LOCATION		¢ PIER 4	0.2	0.4	0.5	0.6	0.8	¢ BEARING FWD. ABUT.
SCREED LINE	STATION	460+55.61	460+74.41	460+93.21	461+02.61	461+12.01	461+30.81	461+49.61
LINE 21 TOE OF MEDIAN BARRIER (LEFT)	FINAL DECK ELEVATION	771.77	771.60	771.40	771.29	771.18	770.94	770.67
	SCREED ELEVATION	771.77	771.62	771.44	771.33	771.22	770.96	770.67
LINE 22	FINAL DECK ELEVATION	771.80	771.62	771.43	771.32	771.21	770.97	770.70
	SCREED ELEVATION	771.80	771.64	771.47	771.36	771.25	770.99	770.70
LINE 23 PROFILE GRADE (NORTHBOUND)	FINAL DECK ELEVATION	771.90	771.73	771.54	771.43	771.32	771.07	770.81
	SCREED ELEVATION	771.90	771.75	771.58	771.47	771.36	771.09	770.81
LINE 24	FINAL DECK ELEVATION	771.92	771.75	771.55	771.45	771.33	771.09	770.83
	SCREED ELEVATION	771.92	771.77	771.59	771.49	771.37	771.11	770.83
LINE 25	FINAL DECK ELEVATION	772.05	771.88	771.68	771.58	771.46	771.22	770.95
	SCREED ELEVATION	772.05	771.90	771.72	771.62	771.50	771.24	770.95
LINE 26	FINAL DECK ELEVATION	772.17	772.00	771.81	771.70	771.59	771.34	771.08
	SCREED ELEVATION	772.17	772.02	771.85	771.74	771.63	771.36	771.08
LINE 27 (PHASE CONST. JOINT 11)	FINAL DECK ELEVATION	772.21	772.04	771.84	771.73	771.62	771.38	771.11
	SCREED ELEVATION	772.21	772.06	771.88	771.77	771.66	771.40	771.11
LINE 28 (PHASE CONST. JOINT 1)	FINAL DECK ELEVATION	772.26	772.08	771.89	771.78	771.67	771.43	771.16
	SCREED ELEVATION	772.26	772.10	771.93	771.82	771.71	771.45	771.16
LINE 29	FINAL DECK ELEVATION	772.28	772.10	771.91	771.80	771.69	771.45	771.18
	SCREED ELEVATION	772.28	772.12	771.95	771.84	771.73	771.47	771.18
LINE 30	FINAL DECK ELEVATION	772.15	771.98	771.78	771.67	771.56	771.32	771.05
	SCREED ELEVATION	772.15	772.00	771.82	771.71	771.60	771.34	771.05
LINE 31	FINAL DECK ELEVATION	772.02	771.85	771.66	771.55	771.44	771.19	770.93
	SCREED ELEVATION	772.02	771.87	771.70	771.59	771.48	771.21	770.93
LINE 32	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 33	FINAL DECK ELEVATION	771.92	771.75	771.55	771.45	771.34	771.10	770.84
	SCREED ELEVATION	771.92	771.77	771.59	771.49	771.38	771.12	770.84
LINE 34	FINAL DECK ELEVATION	-	-	-	-	-	-	-
	SCREED ELEVATION	-	-	-	-	-	-	-
LINE 35	FINAL DECK ELEVATION	771.81	771.64	771.45	771.35	771.24	771.00	770.74
	SCREED ELEVATION	771.81	771.66	771.49	771.39	771.28	771.02	770.74
LINE 36 TOE OF PARAPET (RIGHT)	FINAL DECK ELEVATION	771.78	771.62	771.43	771.32	771.21	770.98	770.72
	SCREED ELEVATION	771.78	771.64	771.47	771.36	771.25	771.00	770.72

NOTES:

- FOR ADDITIONAL NOTES, TYPICAL CROSS SECTION AND SCREED DETAILS, SEE SHEET 40/55.
- FOR SCREED ELEVATIONS, SPANS 1, 2, 3 AND 4, SEE SHEETS 44/55 AND 45/55.
- FOR TABLE OF ANTICIPATED DEAD LOAD DEFLECTIONS, SEE SHEET 43/55.

LEGEND:

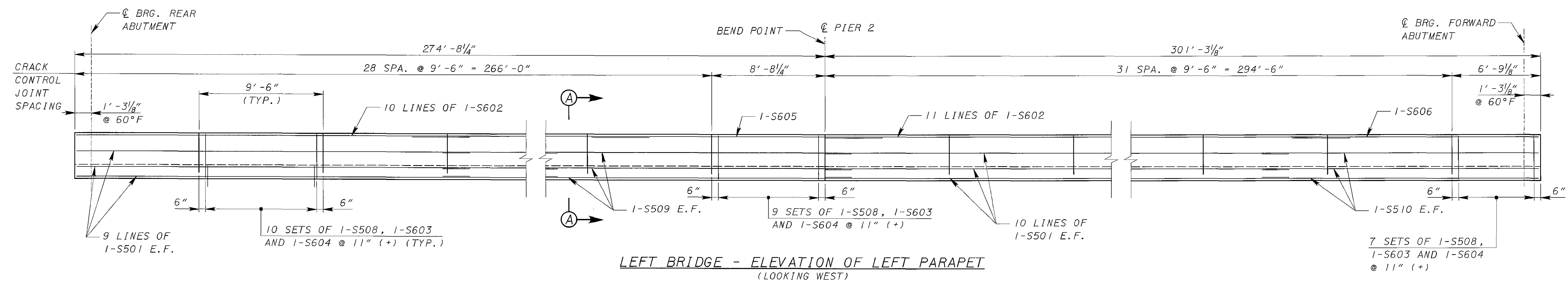
- * FACE OF BARRIER TAPERS FROM 96.4' LEFT OF ¢ AT STA 455+74.69 TO 68' LEFT OF ¢ AT STA 458+49.67
- ** SUPERELEVATION TRANSITION BEGINS AT STA. 456+79.00, RAMP N-6
- *** TRANSITION ENDS AT STA. 458+19.80, I.R. 75
- Δ VARIES .0199 TO .016, STA. 457+43.00 TO 457+73.88



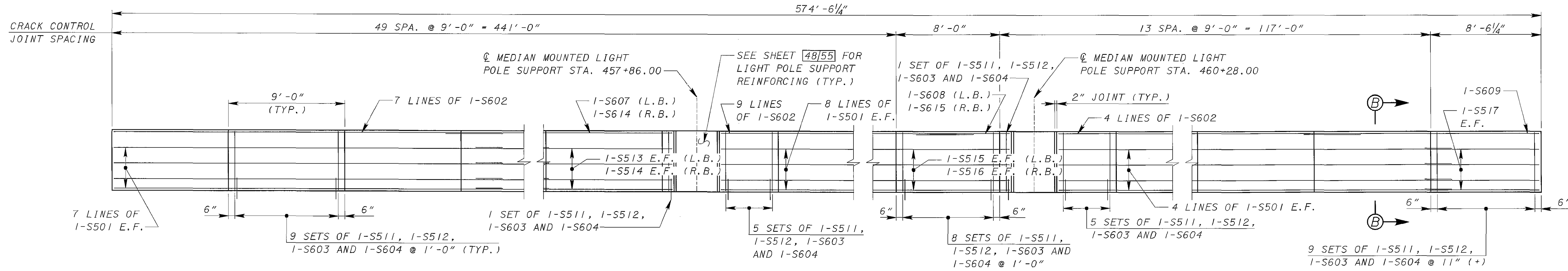
CROSS SLOPE TRANSITION DIAGRAM

(RAMP ACCELERATION LANE BEGINS TAPER AT STA 452+85.40 FROM 25' TO 0' AT A RATE OF 50:1)

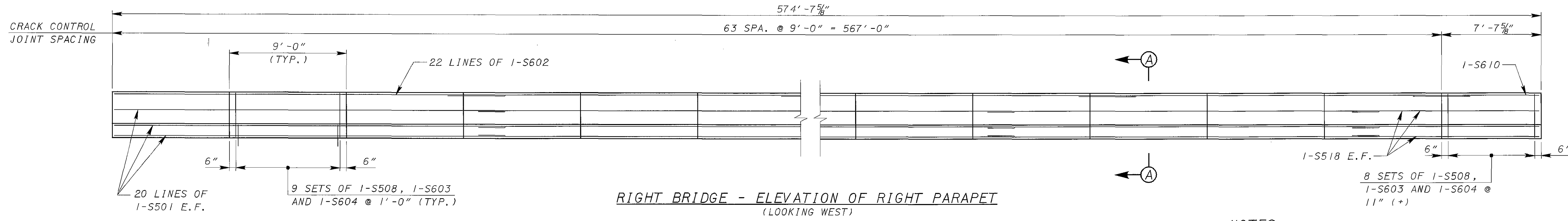
DESIGN AGENCY: **Tran Systems**
 DATE: 5/31/05
 REVIEWED: NFF
 DRAWN: BTA
 DESIGNED: BTA
 CHECKED: JDH
 STRUCTURE FILE NUMBER: 5708710
 REVISION: 5708710
 BRIDGE NO. MOT-75-1523
 I.R-75 OVER THE GREAT MIAMI RIVER
 SCREED ELEVATIONS - RIGHT BRIDGE SPAN 5
 (PRESTRESSED CONCRETE OPTION)
 MOT-75-14.60
 PID 23828
 46/55
 281/314



LEFT BRIDGE - ELEVATION OF LEFT PARAPET
 (LOOKING WEST)



RIGHT BRIDGE - ELEVATION OF MEDIAN BARRIER
 (LOOKING WEST)
 (MEDIAN BARRIER ON LEFT BRIDGE SIMILAR, EXCEPT AS NOTED)



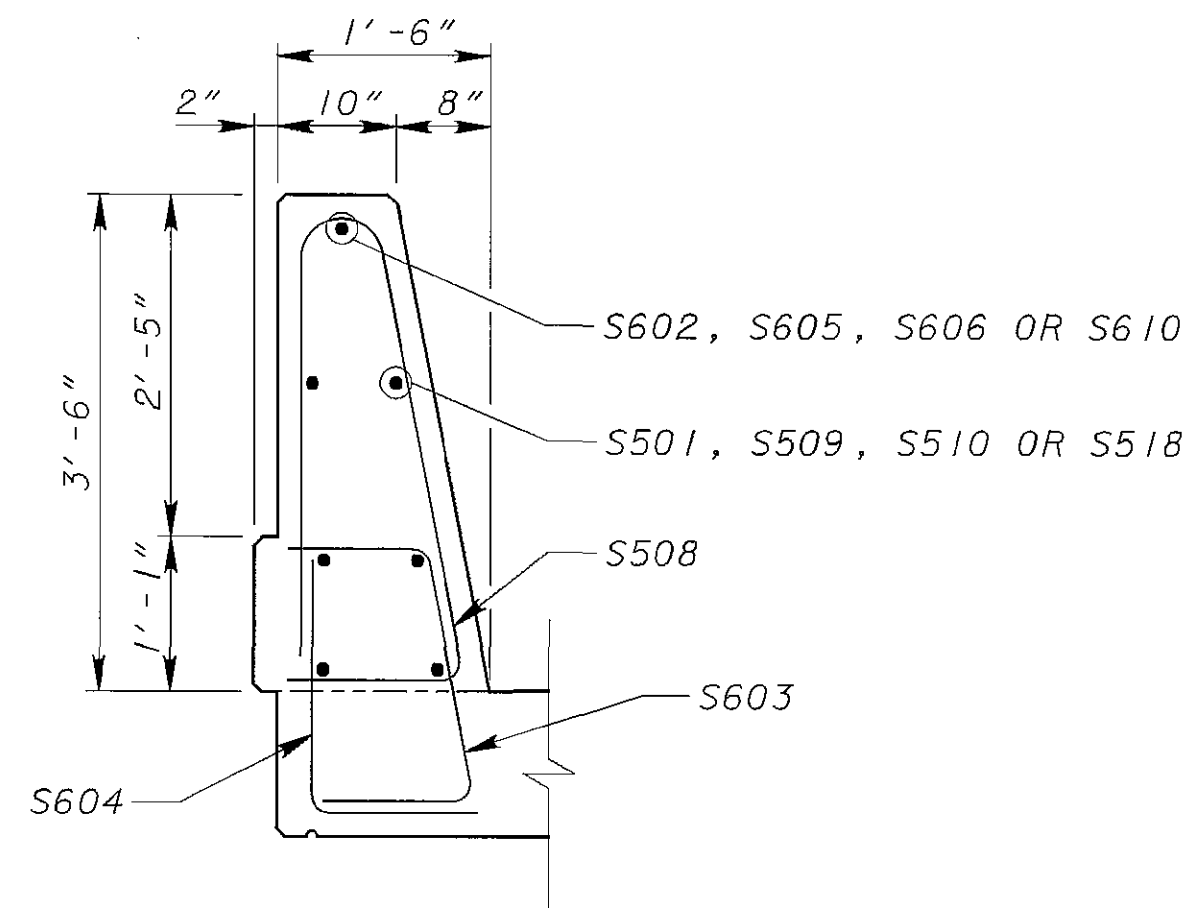
RIGHT BRIDGE - ELEVATION OF RIGHT PARAPET
 (LOOKING WEST)

BAR	REQUIRED LAP LENGTH
#5	2'-0"
#6	4'-4"

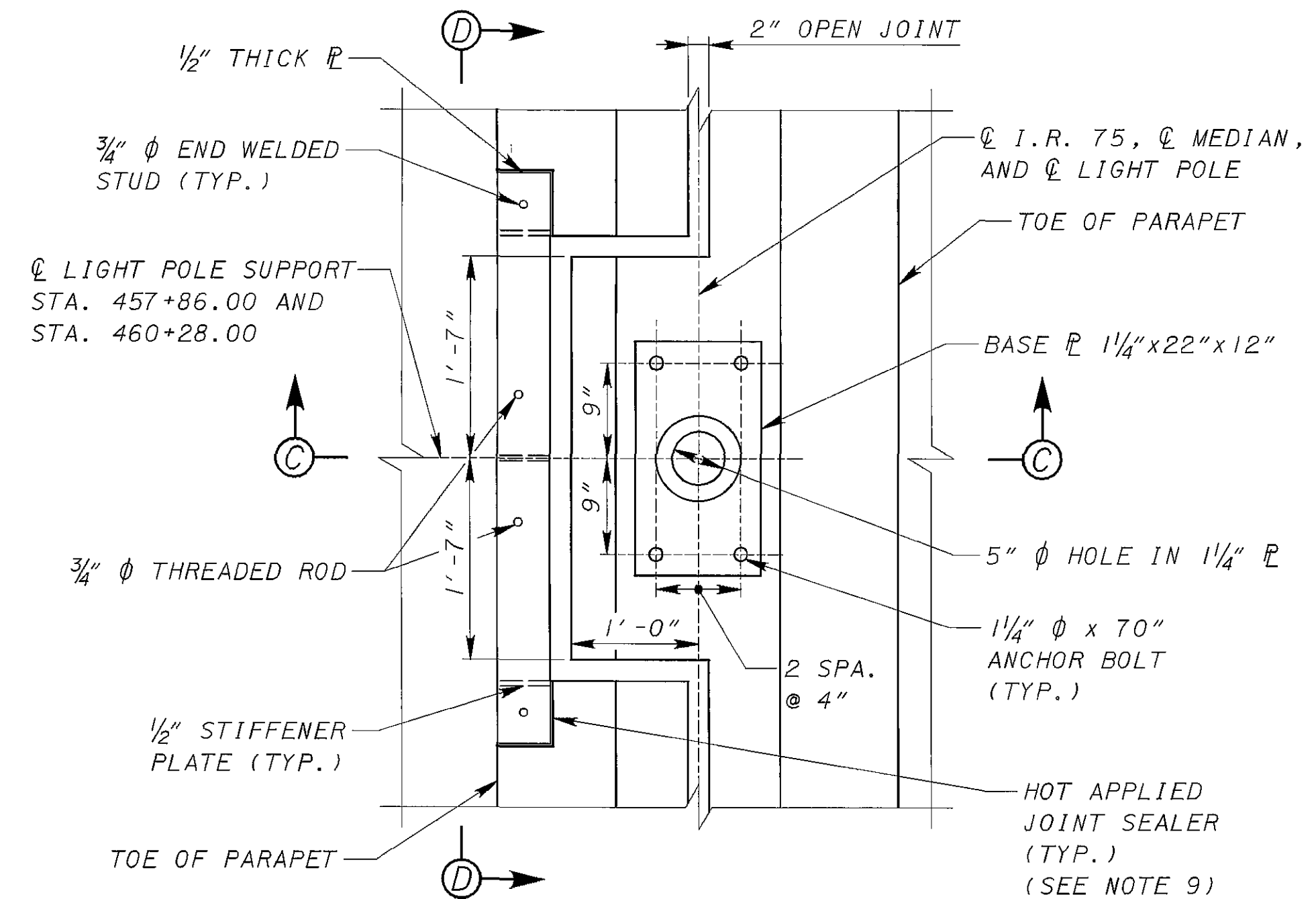
LEGEND:
 R.B. = RIGHT BRIDGE
 L.B. = LEFT BRIDGE

- NOTES:**
- FOR SLAB PLAN, SEE SHEET [39/55].
 - FOR PARAPET SECTIONS AND DETAILS, SEE SHEET [48/55].
 - FOR REINFORCING STEEL LIST, SEE SHEET [55/55].
 - FOR ADDITIONAL PARAPET DETAILS AND NOTES, SEE ODOT DRAWING SBR-1-99.
 - FOR CONTROL JOINTS FOR MEDIAN BARRIER, REFER TO DETAIL AND NOTES ON ODOT DRAWING SBR-1-99.

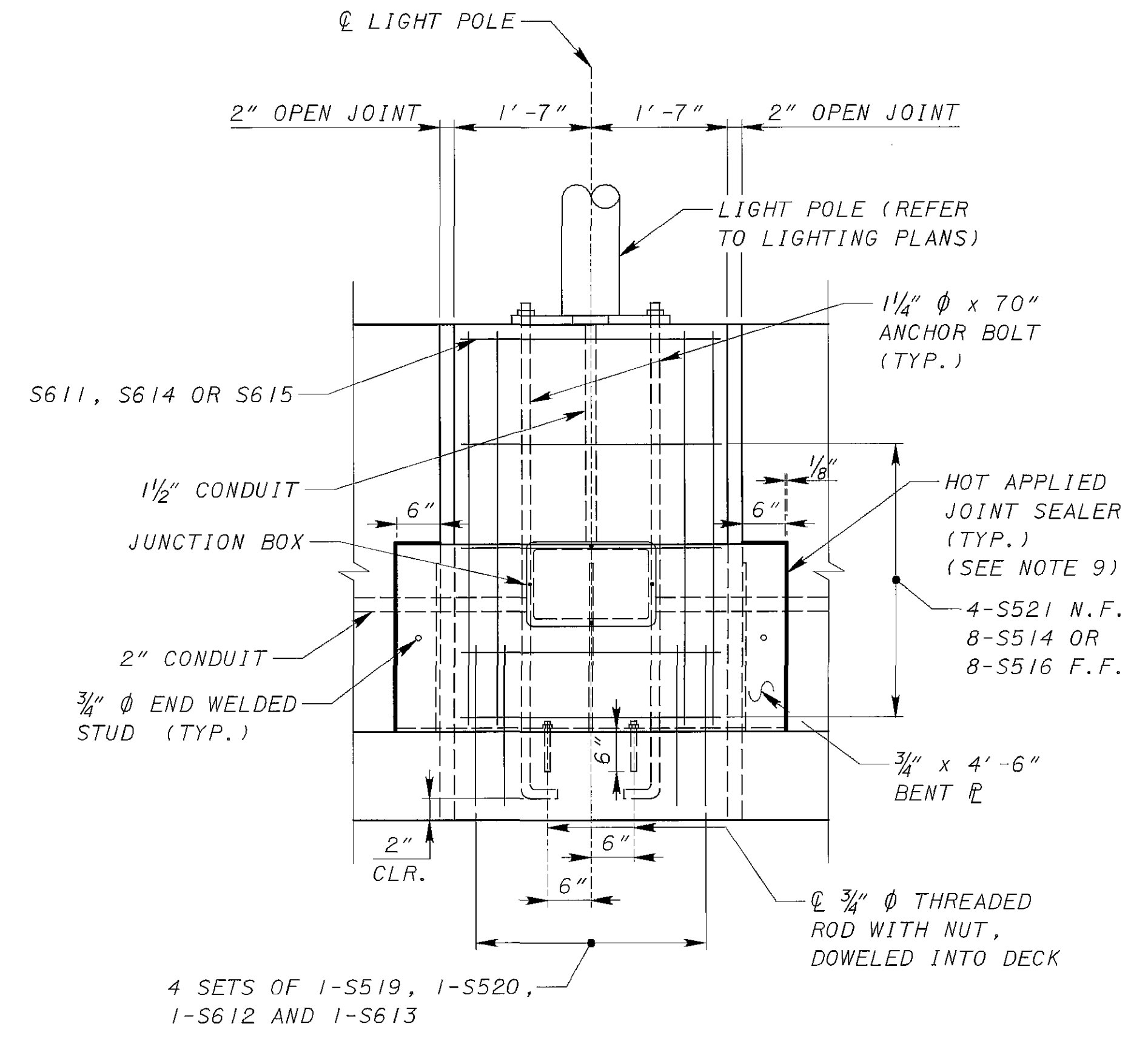
Plotted By: eamcck Date: 7/27/2007 Time: 3:42:25 PM
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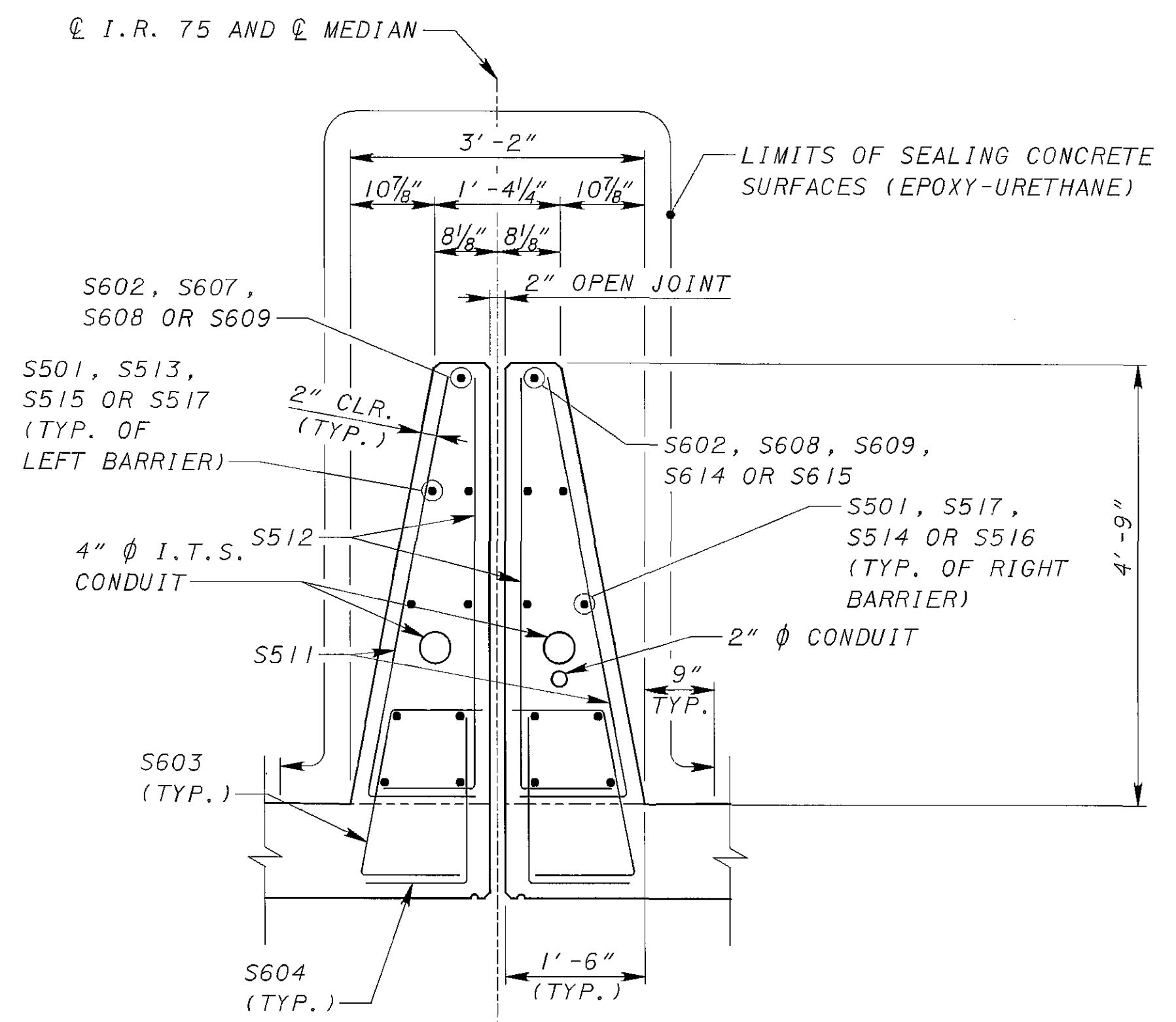
SECTION A-A



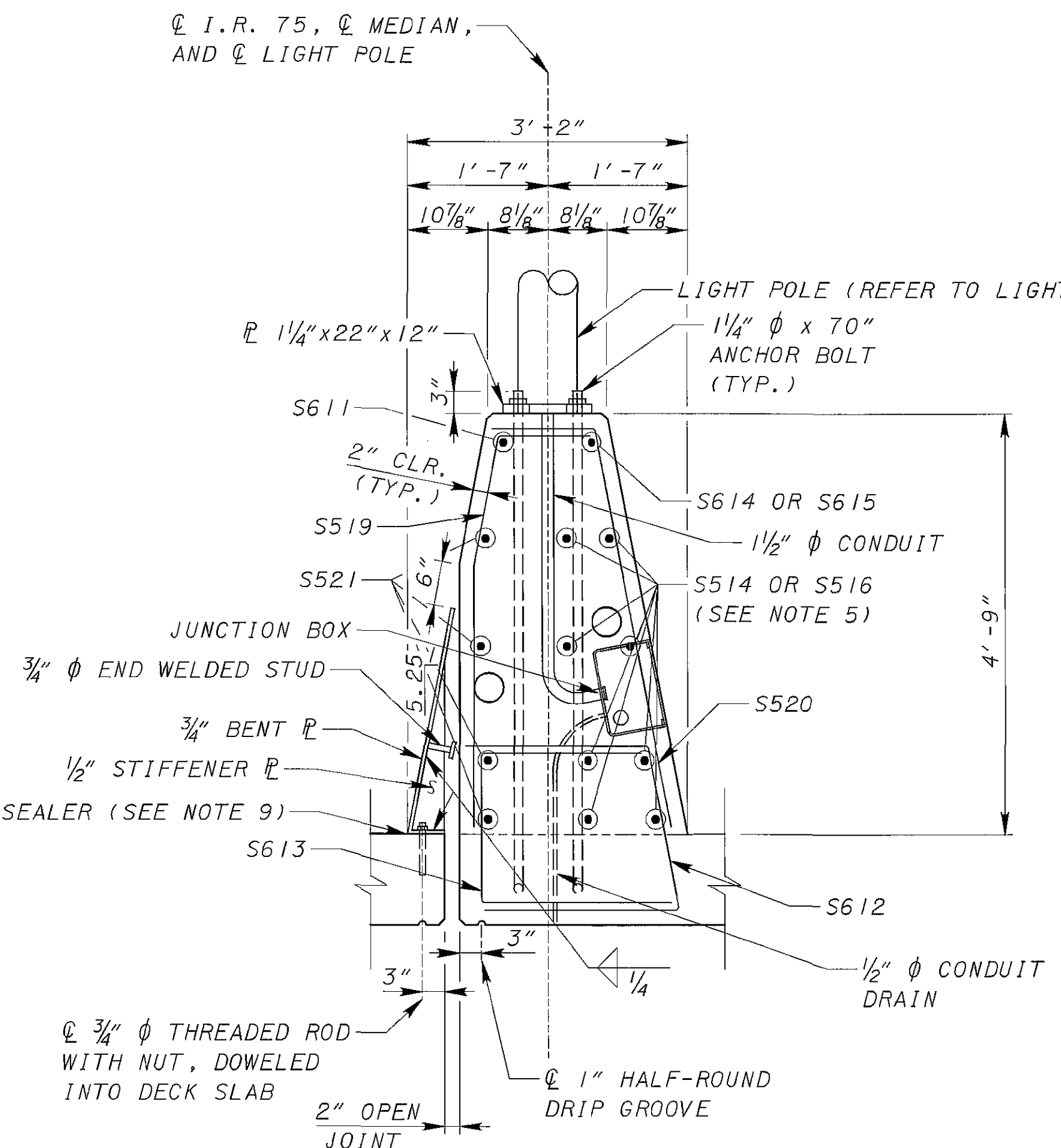
MEDIAN BARRIER DETAIL AT LIGHT POLE SUPPORT PLAN



VIEW D-D (DECK REINFORCEMENT NOT SHOWN)



SECTION B-B (DECK REINFORCEMENT NOT SHOWN)

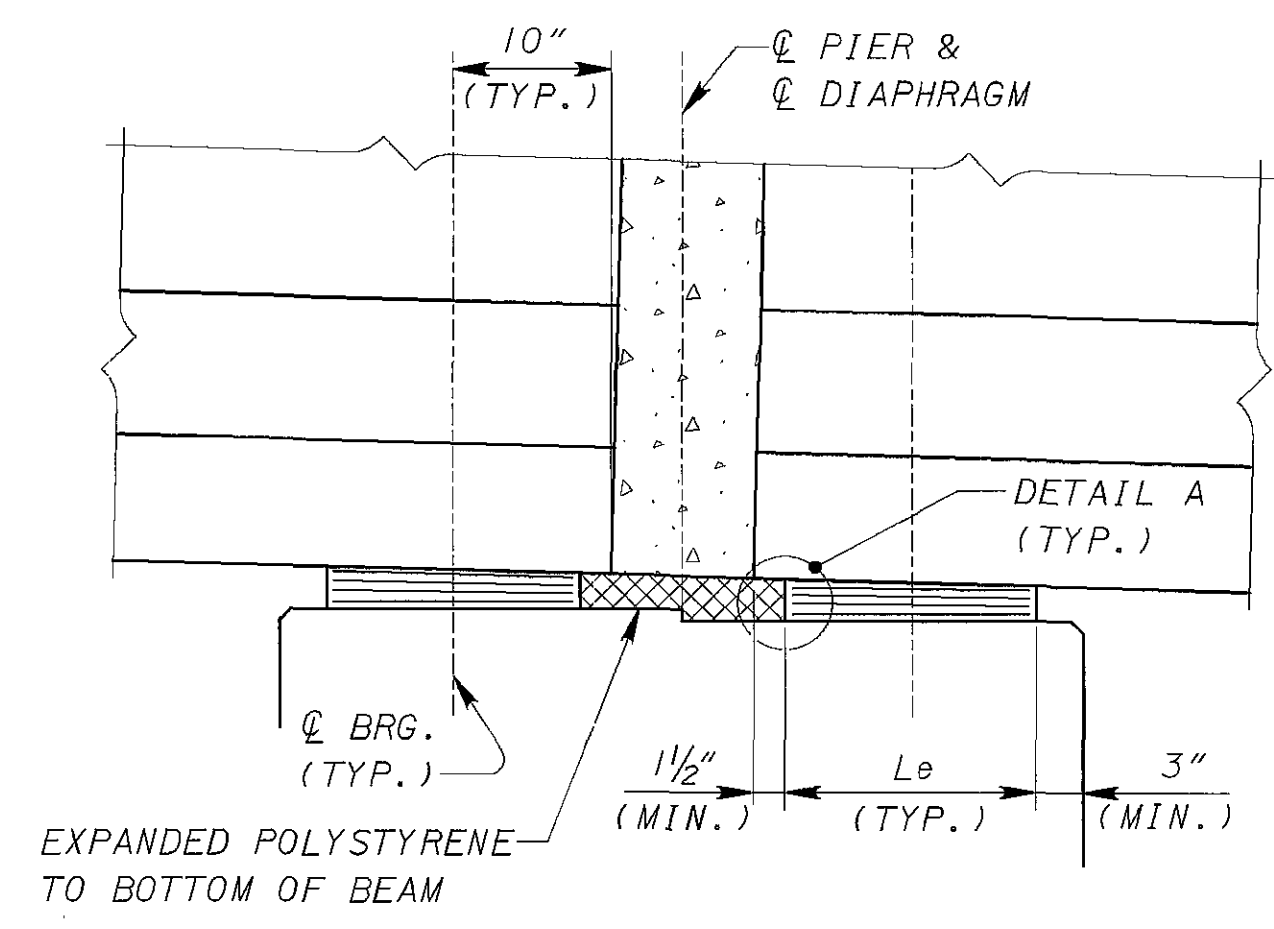
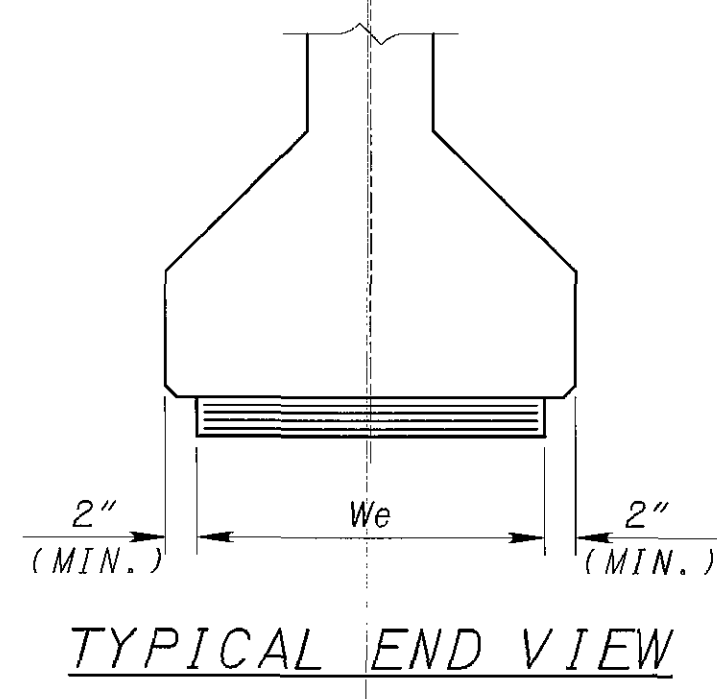
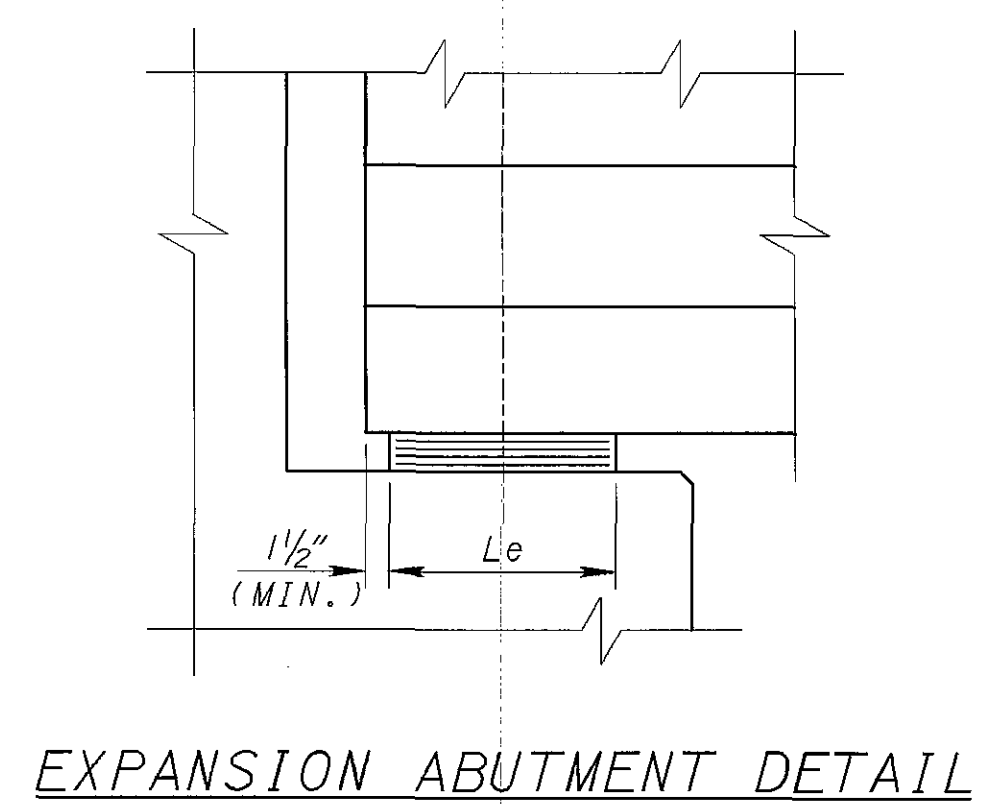
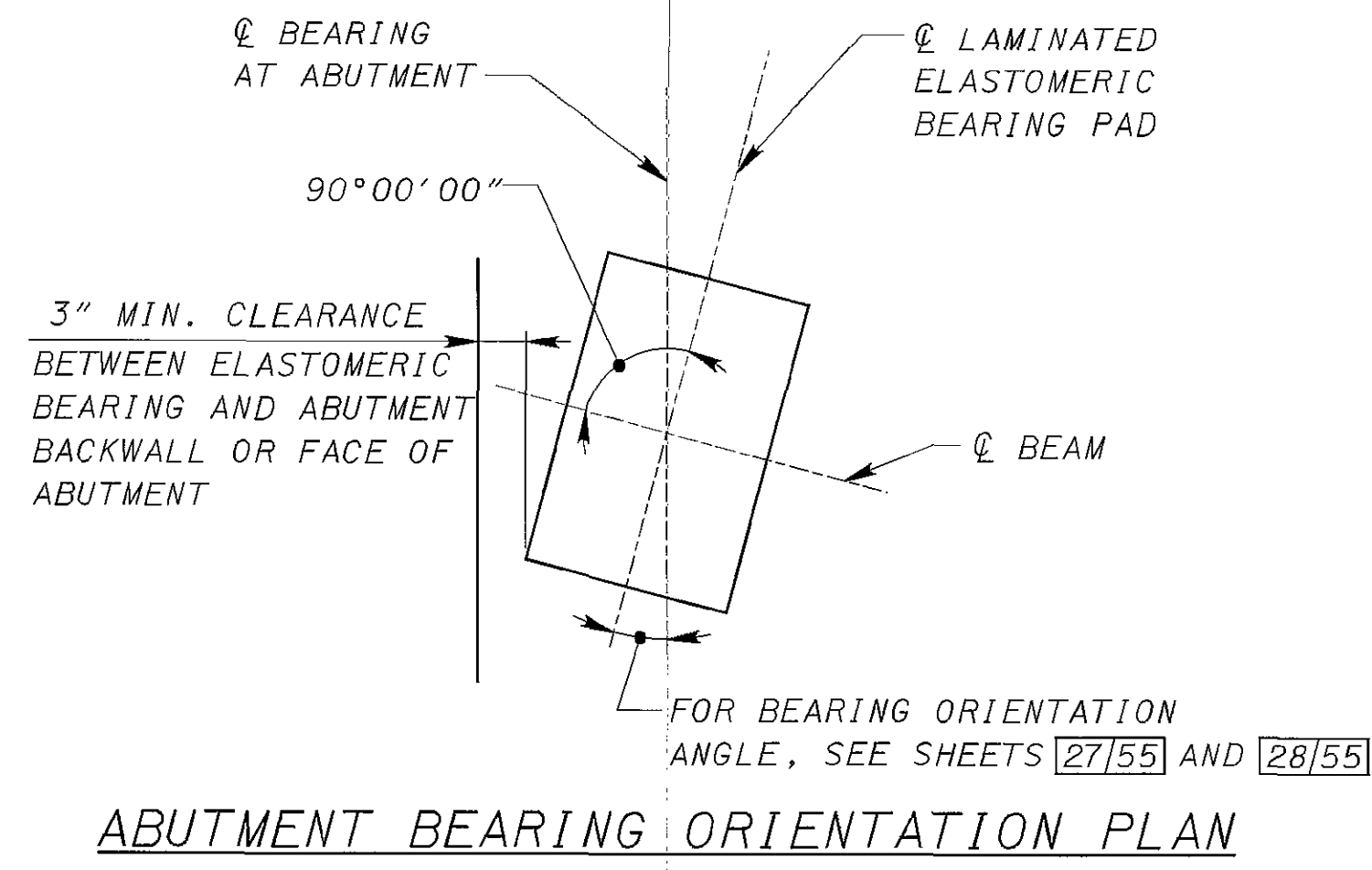


SECTION C-C (DECK REINFORCEMENT NOT SHOWN)

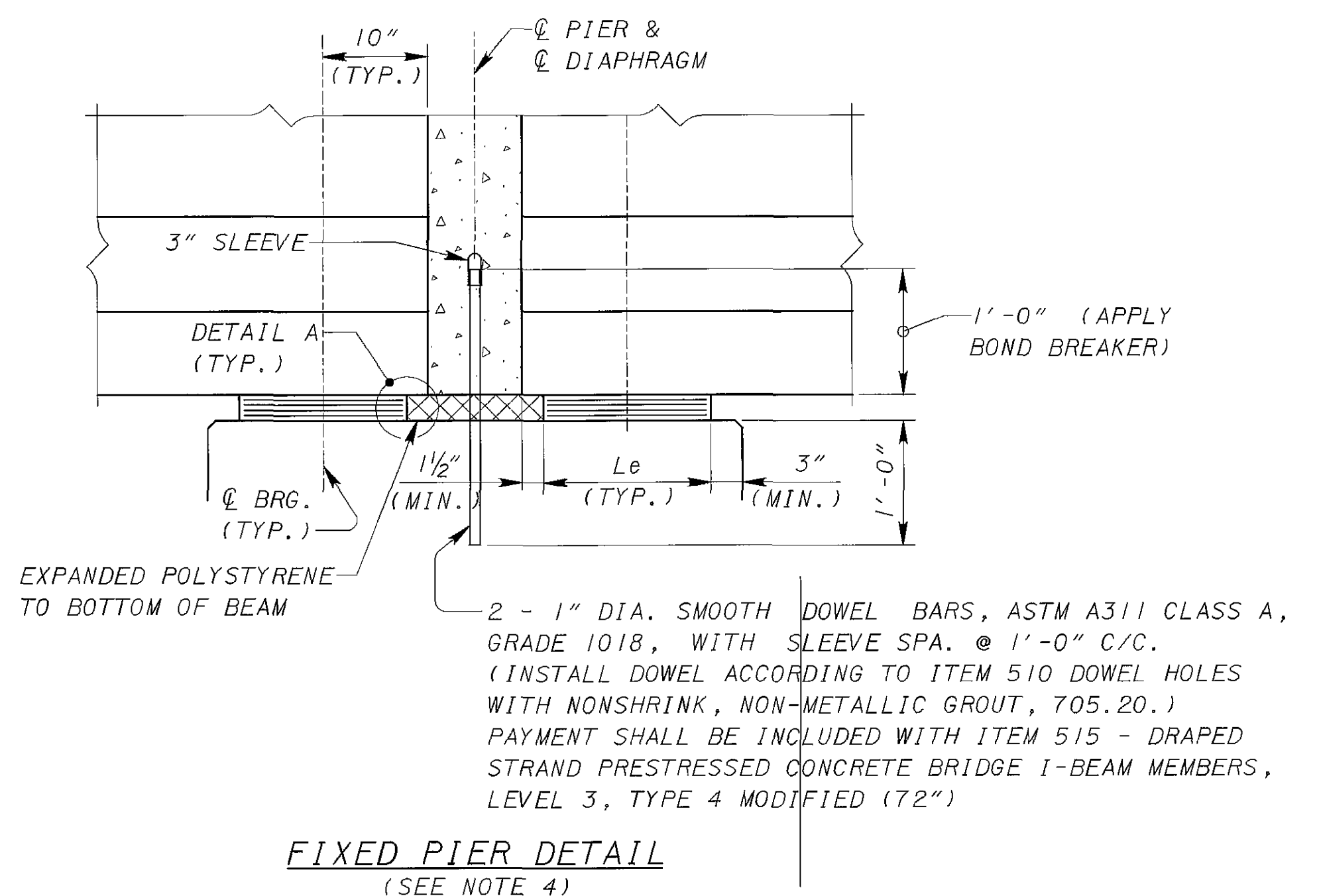
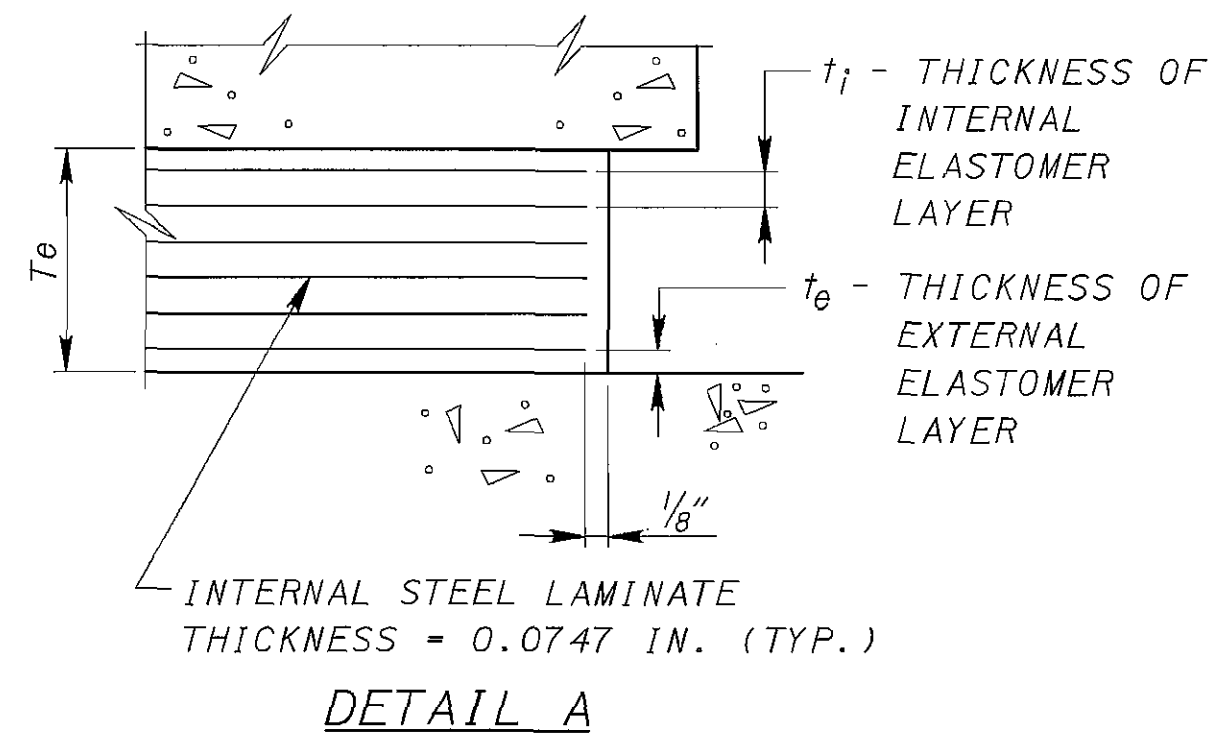
- NOTES:**
- FOR PARAPET DETAILS, SEE SHEET 47/55.
 - FOR SLAB PLAN, SEE SHEET 38/55.
 - FOR REINFORCING STEEL LIST, SEE SHEET 55/55.
 - FOR ADDITIONAL PARAPET DETAILS AND NOTES, SEE ODOT STANDARD DRAWING SBR-1-99.
 - FIELD CUTTING OF LONGITUDINAL REINFORCING BAR AT THE JUNCTION BOX SHALL BE INCLUDED WITH ITEM 509, REINFORCING STEEL, FOR PAYMENT.
 - THE 1 1/4" x 70" ANCHOR BOLTS, NUTS, BASE PLATE AND JUNCTION BOX SHALL BE INCLUDED WITH ITEM 625 FOR PAYMENT. (SEE LIGHTING PLAN)
 - ALL STRUCTURAL STEEL SHALL BE GALVANIZED ACCORDING TO 711.02. GALVANIZING OF STEEL, BASE PLATE, 1 1/4" ANCHOR BOLTS AND JUNCTION BOX SHALL BE INCLUDED WITH ITEM 625 FOR PAYMENT (SEE LIGHTING PLAN).
 - DECK SLAB CONCRETE REQUIRED AT THE LIGHT POLE SUPPORT SHALL BE INCLUDED WITH ITEM 898, CLASS OSC2 CONCRETE, SUPERSTRUCTURE, AS PER PLAN, FOR PAYMENT.
 - HOT APPLIED JOINT SEALER SHALL CONFORM TO 705.04 AND SHALL BE INCLUDED WITH ITEM 898, CLASS OSC2 CONCRETE, SUPERSTRUCTURE, AS PER PLAN, FOR PAYMENT.
 - FOR ADDITIONAL NOTES AND DETAILS, SEE ODOT STANDARD DRAWINGS HL-10.13 AND HL-20.14.
 - FOR CONDUIT TRANSITION FROM BRIDGE MEDIAN BARRIER TO ROADWAY BARRIER, REFER TO ODOT STANDARD DRAWING HL-30.33

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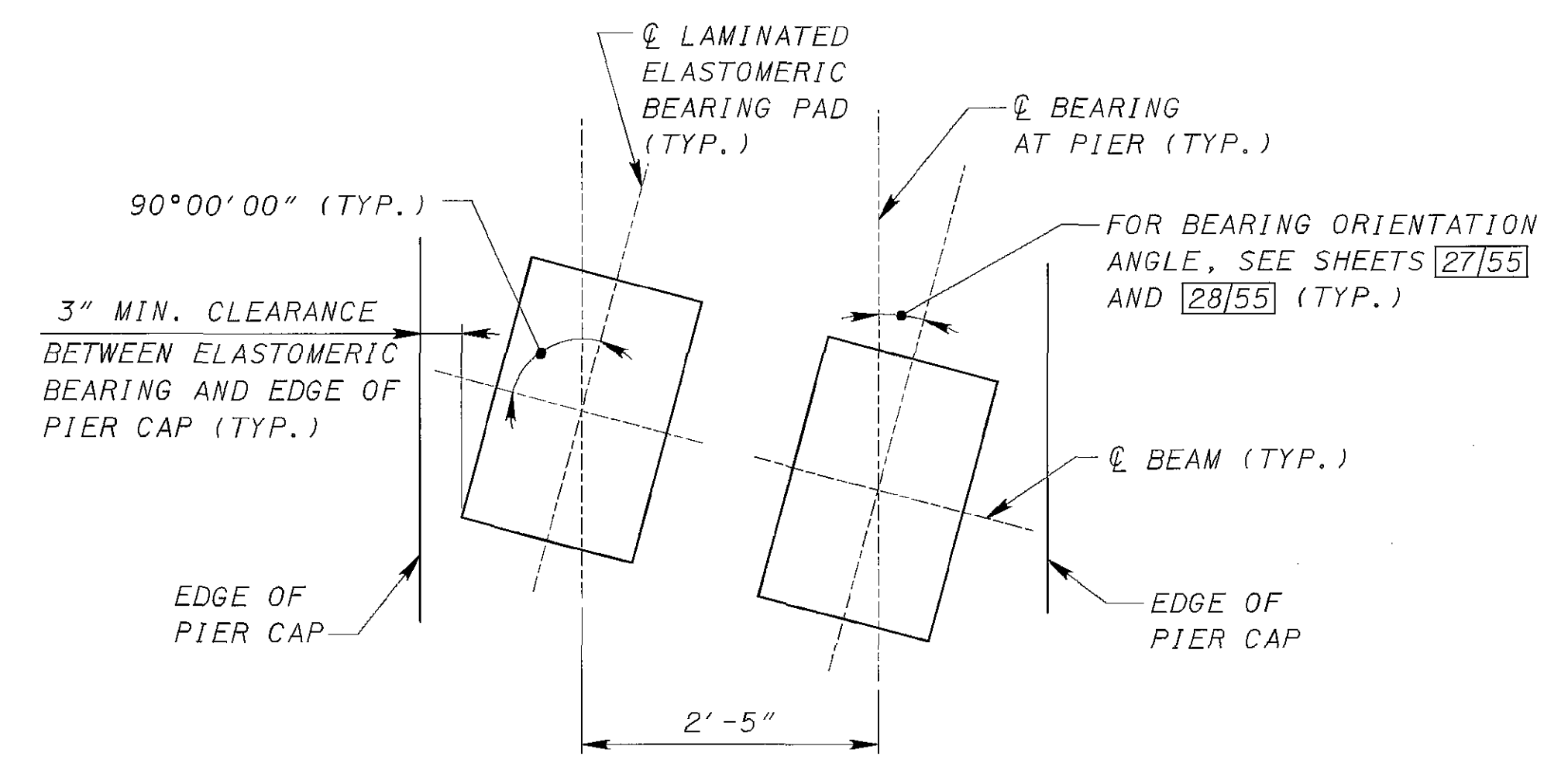
DESIGN AGENCY: **Iran Systems**
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 I.R. 75 OVER THE GREAT MIAMI RIVER
 DESIGNER: BTA
 CHECKED: JDH
 DATE: 05/31/05
 REVIEWED: NFF
 STRUCTURE FILE NUMBER: 5708710
 DRAWN: MLR
 REVISOR: JDH
 MOT-75-14.60
 PID 23828
 48/55
 283/314



EXPANSION PIER DETAIL
(SEE NOTE 4)



FIXED PIER DETAIL
(SEE NOTE 4)



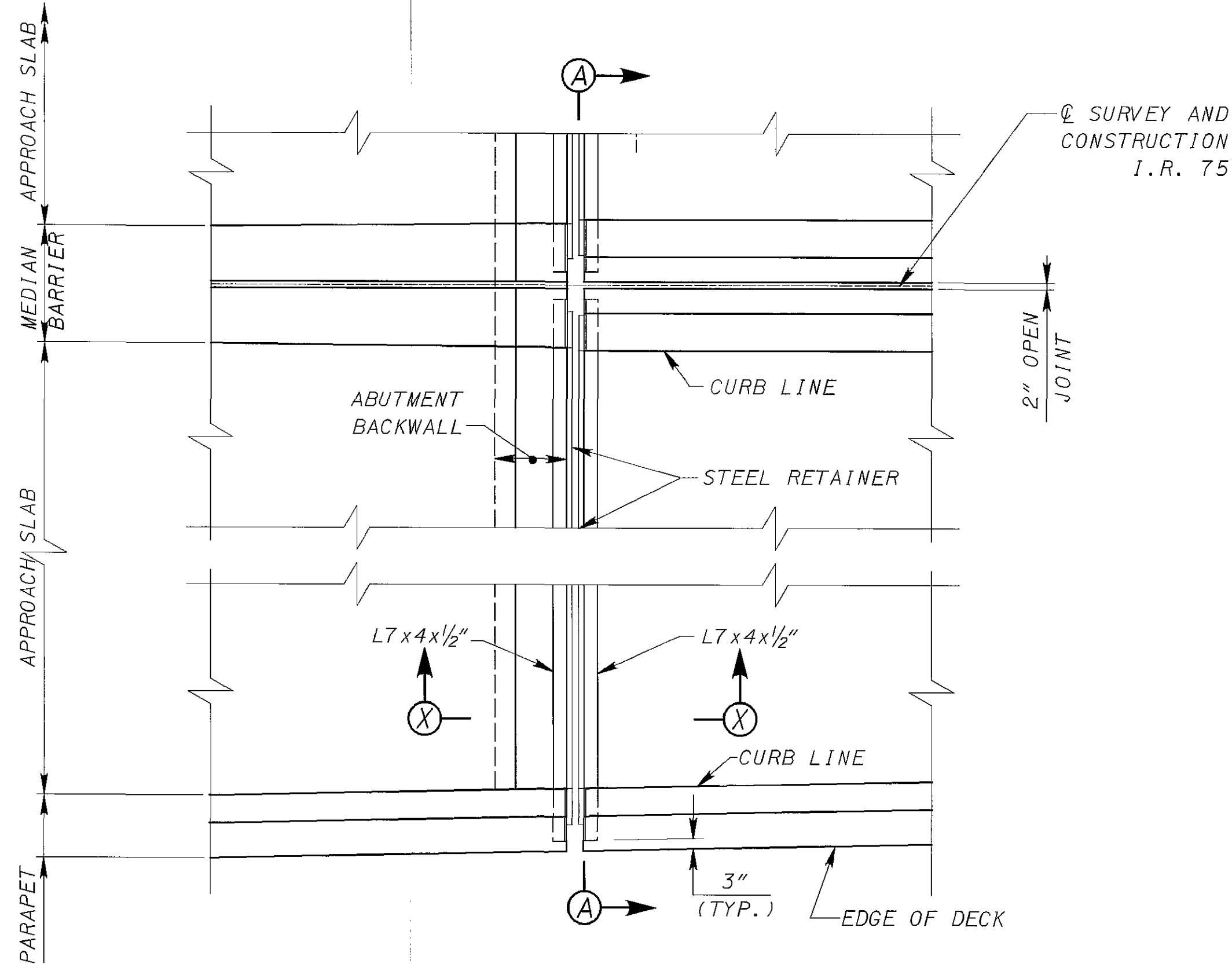
PIER BEARING ORIENTATION PLAN

NOTES:

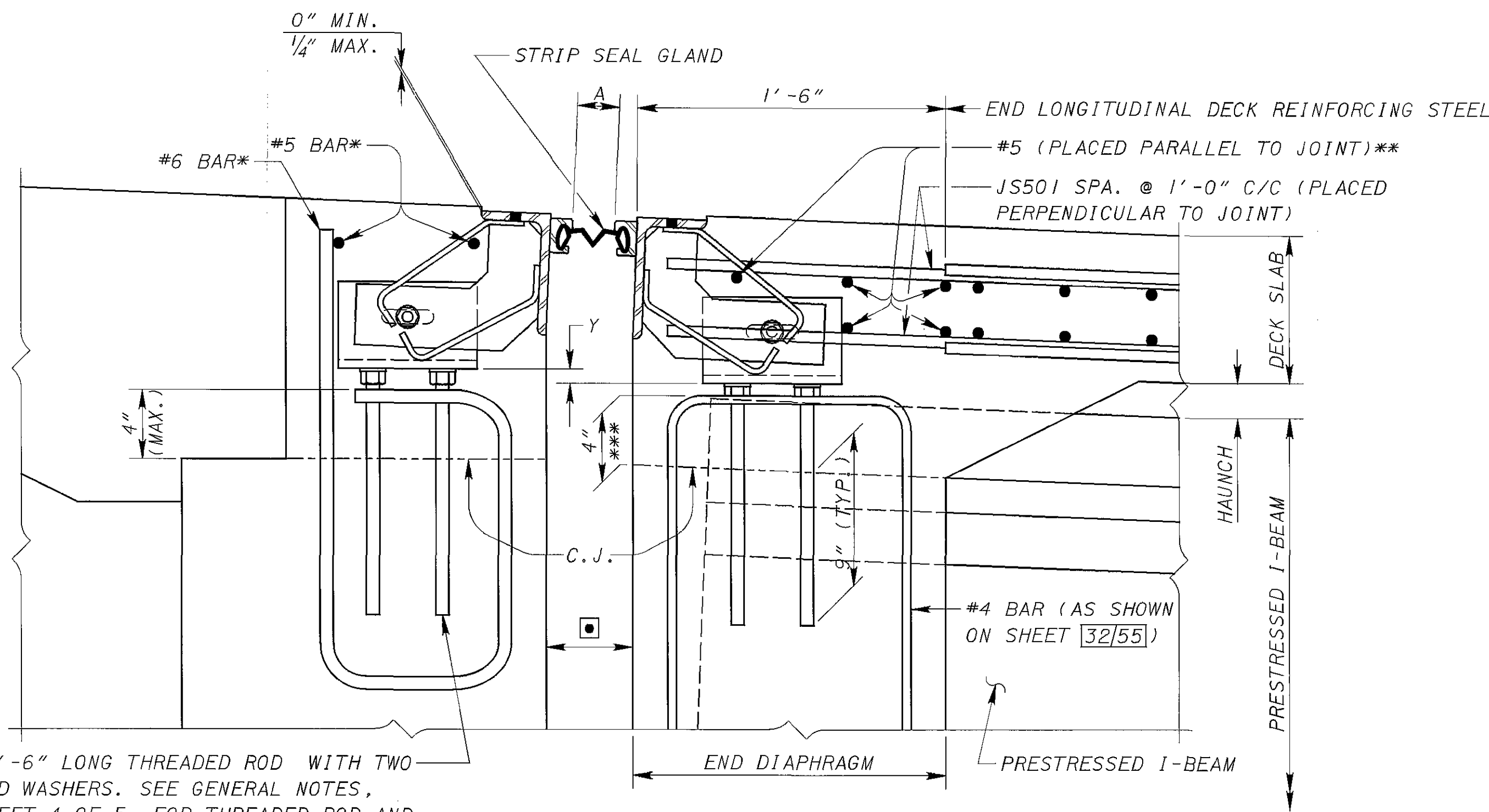
- ELASTOMERIC BEARINGS: THE ELASTOMER SHALL HAVE A HARDNESS OF 60 DUROMETER. THE BEARINGS WERE DESIGNED UNDER DIVISION I, SECTION 14.6.6 (METHOD A) OF THE AASHTO STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES.
- BEARING REPOSITIONING: IF THE CONCRETE IS ERECTED AT AN AMBIENT TEMPERATURE HIGHER THAN 80°F OR LOWER THAN 40°F AND THE BEARING SHEAR DEFLECTION EXCEEDS 1/6 OF THE BEARING HEIGHT AT 60°F ± 10°F, RAISE THE BEAMS TO ALLOW THE BEARINGS TO RETURN TO THEIR UNDEFORMED SHAPE AT 60°F ± 10°F.
- BASIS OF PAYMENT - THE UNIT BID PRICE SHALL INCLUDE ALL MATERIALS LABOR, TESTING, PAINTING AND INCIDENTALS NECESSARY TO FURNISH AND INSTALL LAMINATED ELASTOMERIC BEARINGS. PAYMENT WILL BE MADE AT THE CONTRACT PRICE FOR ITEM 516, EACH, ELASTOMERIC BEARINGS WITH INTERNAL LAMINATES (NEOPRENE).
- REINFORCEMENT IN DIAPHRAGM NOT SHOWN FOR CLARITY.

ELASTOMERIC BEARING DATA													
LOCATION	TYPE	NO. REQ'D	DL (KIP)	LL (KIP) WITHOUT IMPACT	TOTAL LOAD (DL+LL)	Le	We	ti	te	NO. OF ti's	NO. OF te's	NO. INTERNAL LAMINATES	Te
						(in.)	(in.)	(in.)	(in.)				(in.)
REAR ABUT.	EXP	22	166	69	235	13.5	22	0.66	0.38	5	2	6	4.508
PIER No. 1	EXP	42	185	85	270	14	22	0.66	0.38	5	2	6	4.508
PIER No. 2	FIXED	38	170	77	247	10	22	0.42	0.28	3	2	4	2.119
PIER No. 3	EXP	36	130	69	199	12	18	0.36	0.25	7	2	8	3.618
PIER No. 4	EXP	36	134	67	201	13.5	18.5	0.56	0.29	6	2	7	4.463
FWD. ABUT.	EXP	18	115	64	179	15.5	15.5	0.70	0.42	5	2	6	4.888

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PART PLAN AT ABUTMENT
 (REAR ABUTMENT SHOWN, FORWARD ABUTMENT SIMILAR)



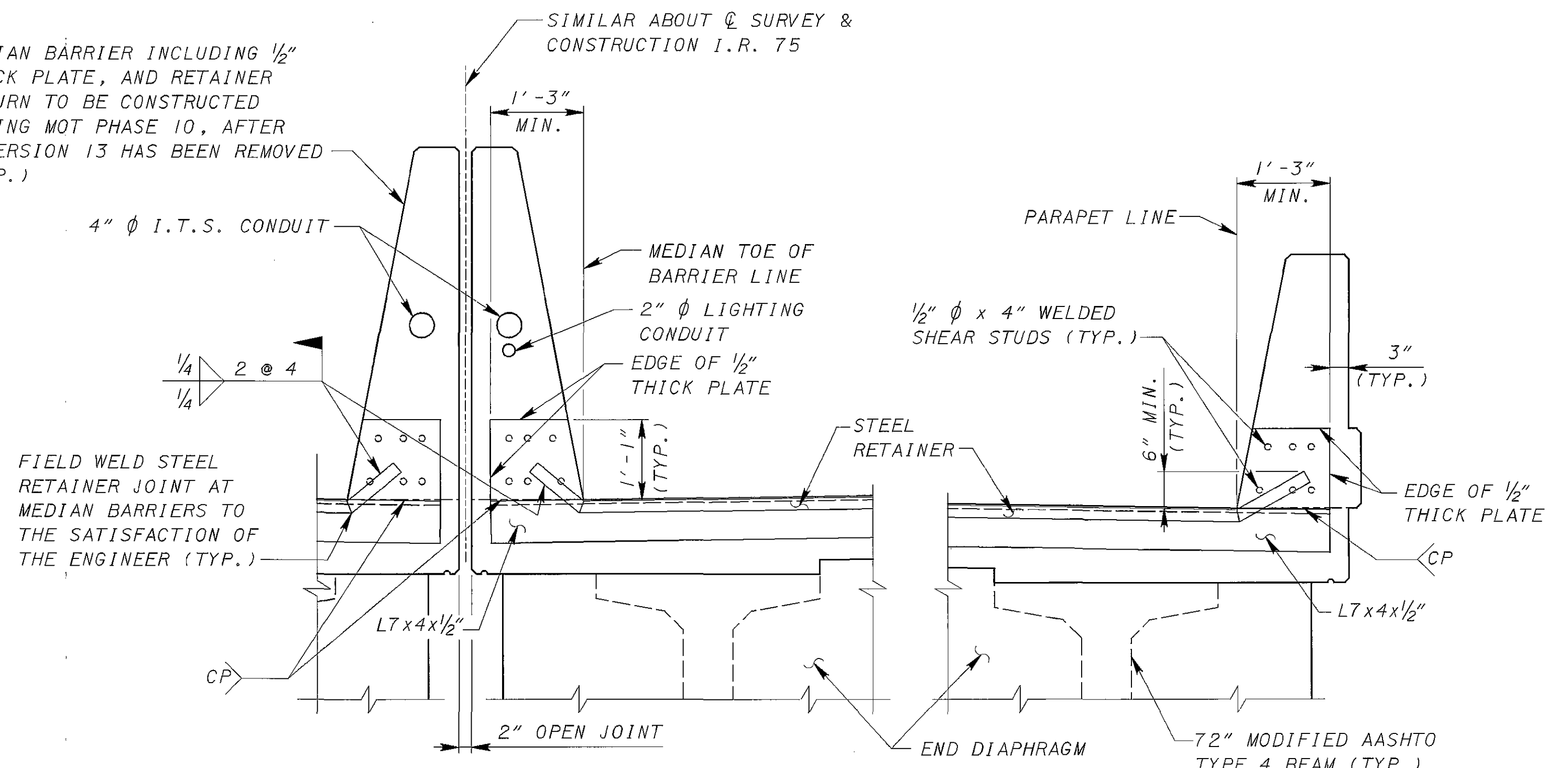
SECTION X-X

3/4" DIA. x 1'-6" LONG THREADED ROD WITH TWO HEX NUTS AND WASHERS. SEE GENERAL NOTES, EXJ-6-06 SHEET 4 OF 5, FOR THREADED ROD AND JOINT SUPPORT & ANCHORAGE REQUIREMENTS. (TYP.)

- - THIS DIMENSION IS THE SUM OF (2xSTEEL RETAINER WIDTH + DIM. A)
- *** - MEASURED TO THE HIGHEST SIDE OF THE DIAPHRAGM
- Y - %GRADE x (DIM. A + 18 1/2")
- %GRADE - INSTANTANEOUS PROFILE GRADE AT THE CENTERLINE OF THE JOINT

- * SEE REAR ABUTMENT SHEET 17/55 AND FORWARD ABUTMENT SHEET 20/55
- ** SEE SLAB PLAN SHEET 38/55

MEDIAN BARRIER INCLUDING 1/2" THICK PLATE, AND RETAINER UPTURN TO BE CONSTRUCTED DURING MOT PHASE 10, AFTER DIVERSION 13 HAS BEEN REMOVED (TYP.)



SECTION A-A

4" STRIP SEAL JOINT WIDTH REAR ABUTMENT		
AMBIENT TEMP.	DIMENSION A	DIMENSION Y
90° F	1 5/16"	+7/16"
80° F	1 1/2"	+7/16"
70° F	1 1/16"	+7/16"
60° F	1 7/8"	+1/2"
50° F	2 1/8"	+1/2"
40° F	2 5/16"	+1/2"
30° F	2 1/2"	+1/2"

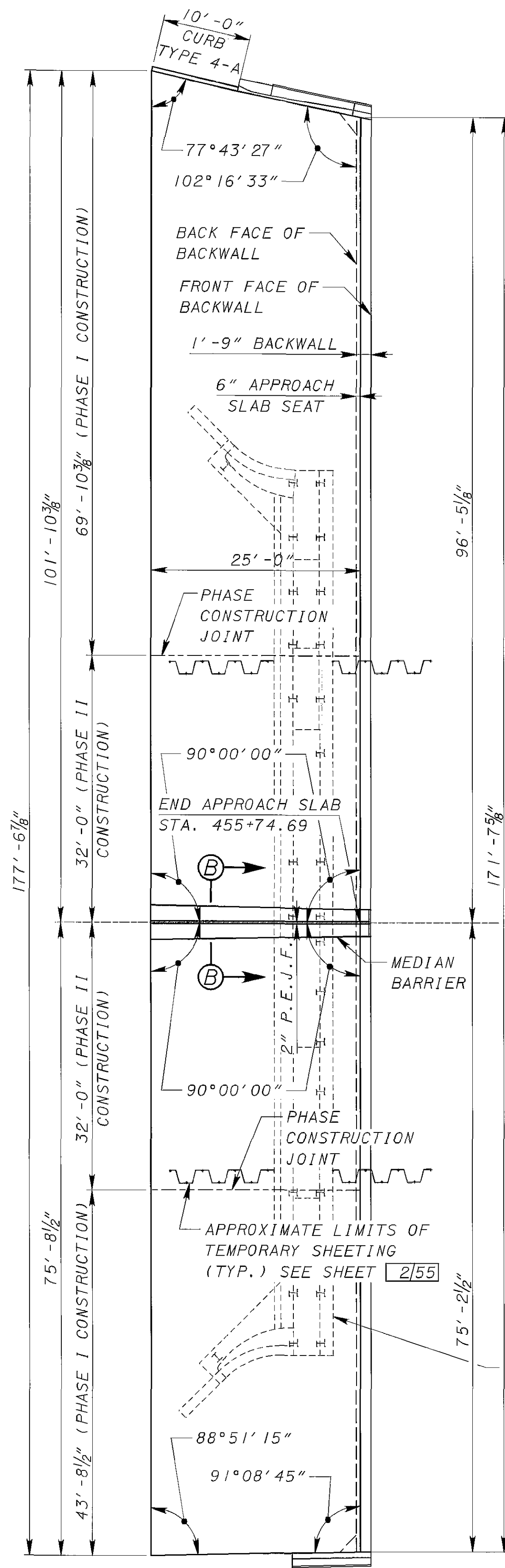
4" STRIP SEAL JOINT WIDTH FORWARD ABUTMENT		
AMBIENT TEMP.	DIMENSION A	DIMENSION Y
90° F	1 1/4"	-5/16"
80° F	1 7/16"	-5/16"
70° F	1 1/16"	-5/16"
60° F	1 7/8"	-5/16"
50° F	2 1/8"	-5/16"
40° F	2 5/16"	-5/16"
30° F	2 9/16"	-5/16"

NOTES:
 MINIMUM JOINT OPENING (DIMENSION "A") AT TIME OF SEAL GLAND INSTALLATION SHALL NOT BE LESS THAN 1 1/2". IF THE JOINT OPENING IS LESS, INSTALLATION SHALL BE POSTPONED UNTIL THE TEMPERATURE DROPS A SUFFICIENT AMOUNT TO ALLOW THE MINIMUM 1 1/2" OPENING.

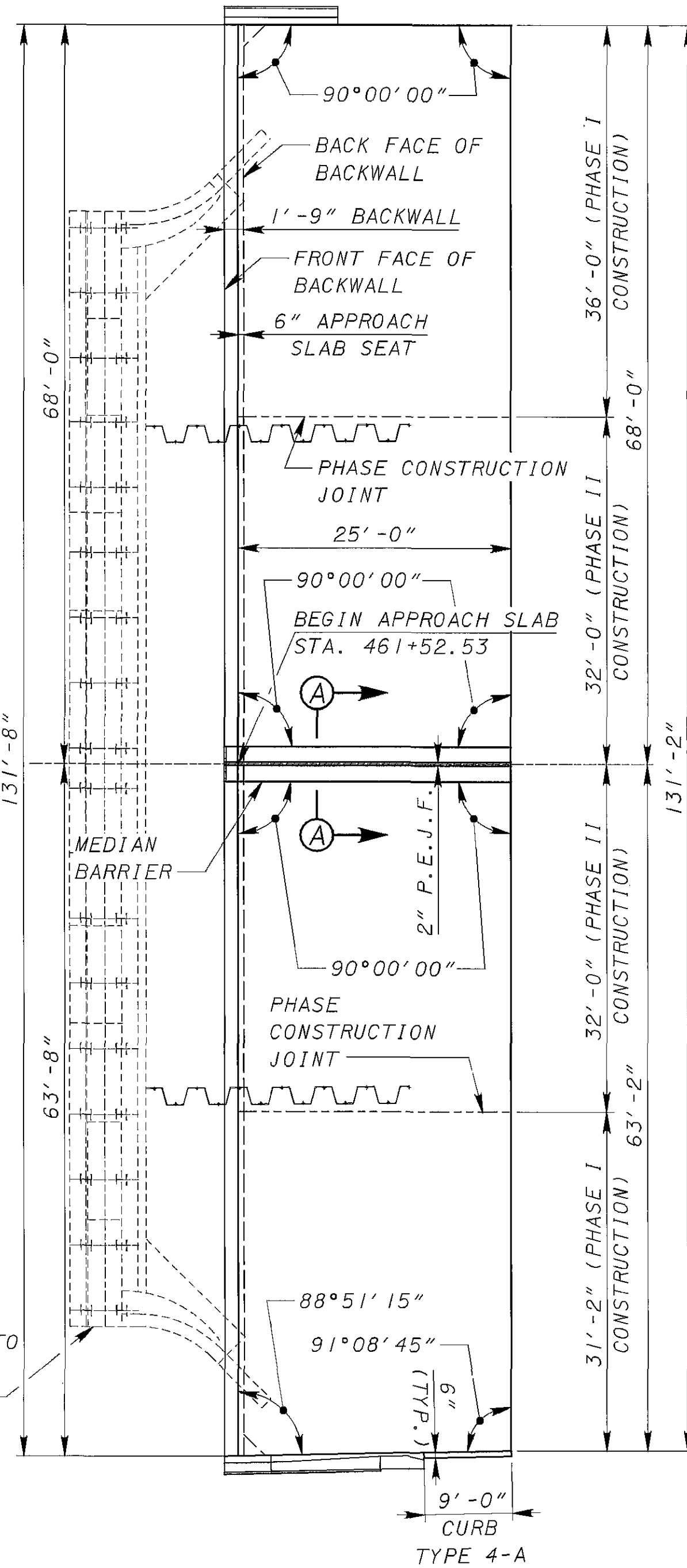
A POSITIVE VALUE FOR DIMENSION Y INDICATES A POSITIVE INSTANTANEOUS PROFILE GRADE LOOKING UPSTATION. A NEGATIVE VALUE FOR DIMENSION Y INDICATES A NEGATIVE INSTANTANEOUS PROFILE GRADE LOOKING UPSTATION.

NOTES:

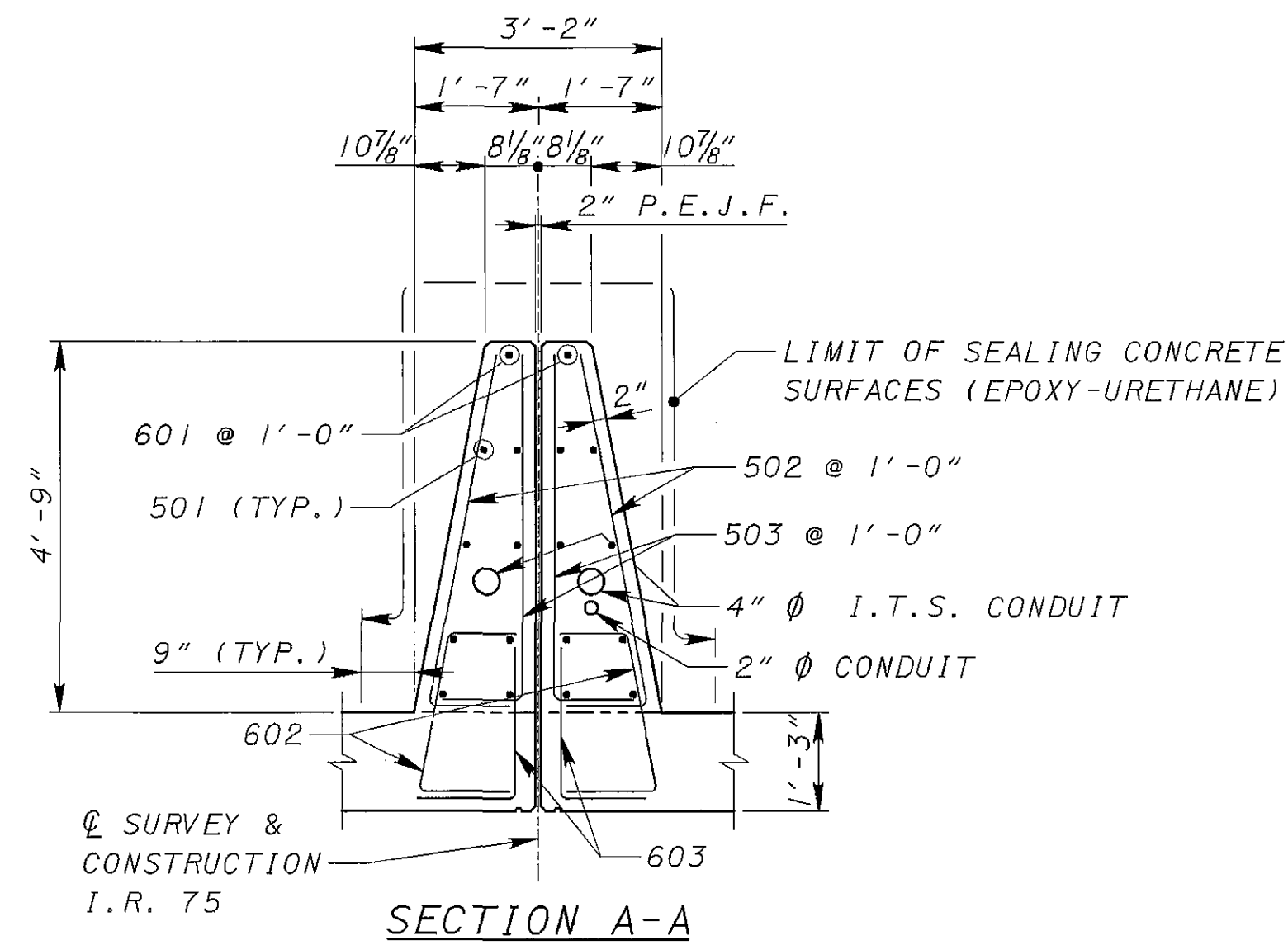
1. FOR ADDITIONAL DETAILS NOT SHOWN, REFER TO ODOT STANDARD BRIDGE DRAWING EXJ-6-06.
2. THE INSTALLATION SEQUENCE OF THE EXPANSION JOINT SHALL FOLLOW THE CONSTRUCTION PROCEDURE ON ODOT STANDARD DRAWING EXJ-6-06, SHEET 4 OF 5 AND AS MODIFIED HEREIN.
3. NO JOINTS IN STRIP SEALS ARE ALLOWED UNLESS APPROVED BY THE DIRECTOR.
4. FOR JOINT TREATMENTS IN RETAINERS AND IN ARMOR STEEL, REFER TO ODOT STANDARD DRAWING EXJ-6-06, SHEET 4 OF 5.



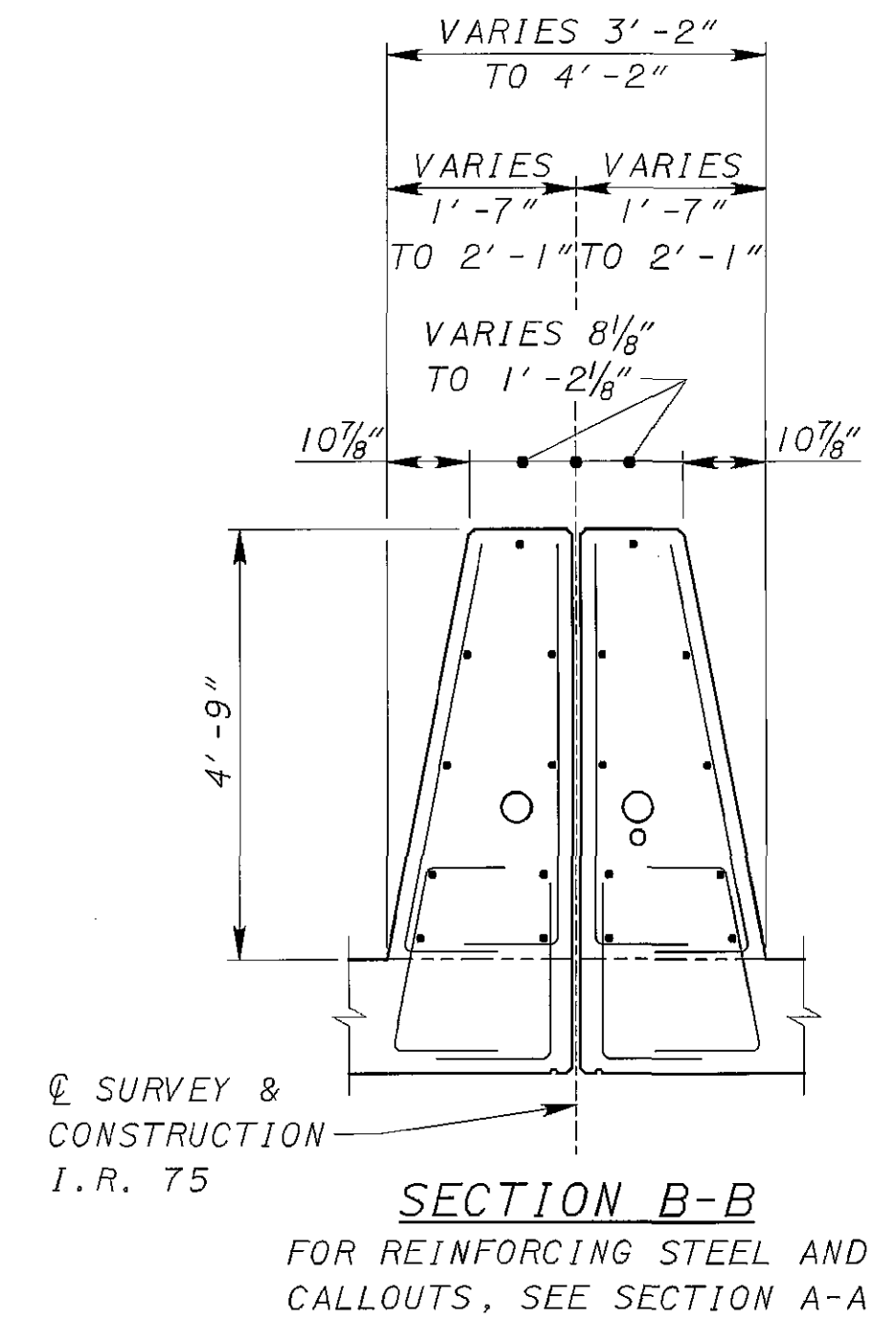
PLAN
REAR APPROACH SLAB



PLAN
FORWARD APPROACH SLAB

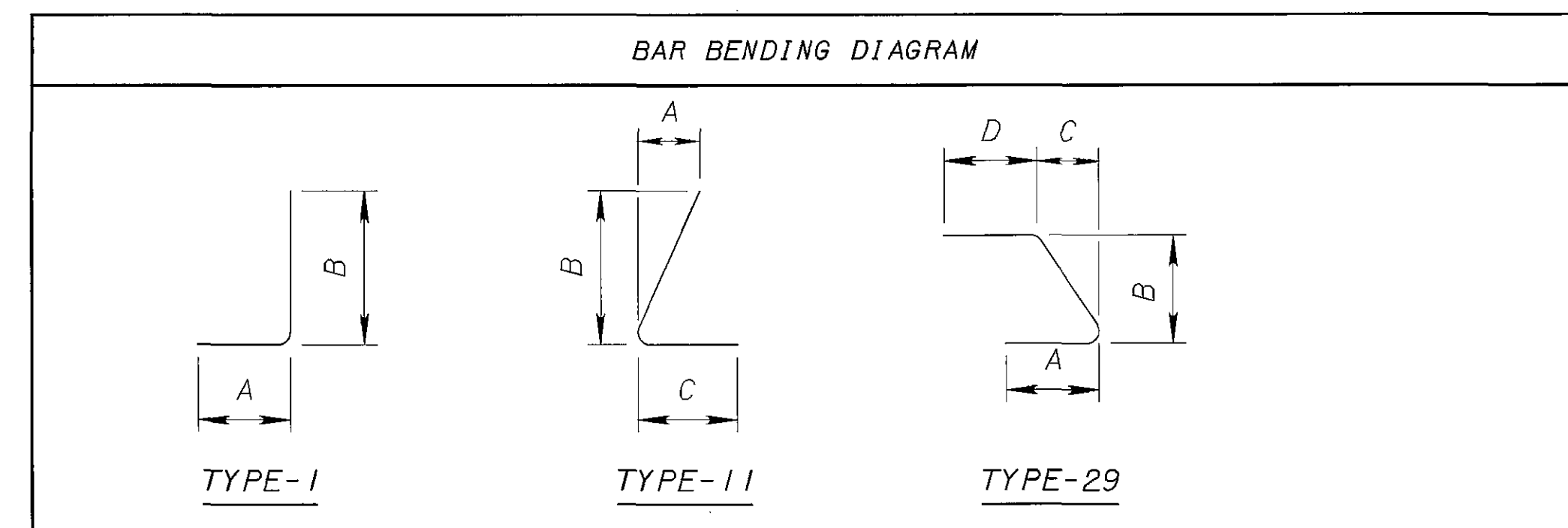


SECTION A-A



SECTION B-B
FOR REINFORCING STEEL AND
CALLOUTS, SEE SECTION A-A

BAR SCHEDULE											
MARK	NUMBER	LENGTH	WEIGHT	TYPE	DIMENSIONS						
					A	B	C	D	E	R	INC.
MEDIAN BARRIER											
501	32	25'-11"	866	STR							
502	108	5'-6"	620	11	10 1/4"	4'-5"	1'-1"				
503	108	5'-5"	610	1	1'-1"	4'-5"					
601	4	25'-11"	156	STR							
602	108	4'-3"	689	29	1'-4"	2'-1"	4 1/2"	1'-1"			
603	108	3'-3"	528	1	1'-4"	2'-1"					
TOTAL WEIGHT OF REINFORCING = 3469 LBS											



NOTES:

- FOR ADDITIONAL NOTES, DETAILS, SECTIONS, AND REINFORCING BARS NOT SHOWN HERE, SEE ODOT STANDARD DRAWING AS-1-81.
- FOR CURB TYPE 4-A, REFER TO ODOT STANDARD DRAWING BP-5.1.
- SEALING OF CONCRETE SURFACES ON MEDIAN PARAPETS IS INCLUDED WITH ITEM 512.
- FOR TEMPORARY SHEETING NOTES, SEE GENERAL PLAN SHEET 255.
- SEE ROADWAY SHEETS FOR ADDITIONAL MEDIAN BARRIER TRANSITION DETAILS.
- FOR ITEMS INCLUDED WITH ITEM 898 - QC/QA CONCRETE, CLASS QSC2, SUPERSTRUCTURE (APPROACH SLAB), 15", AS PER PLAN., SEE STRUCTURE GENERAL NOTES.

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BAR SCHEDULE

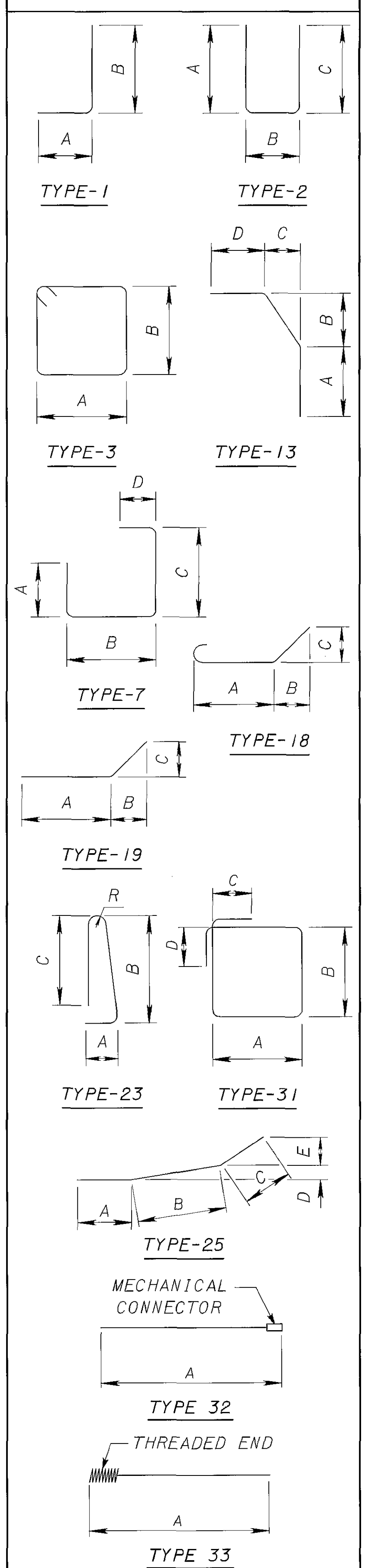
MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS					
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R
REAR ABUTMENT												
RA501	109	64	9'-10"	1118	656	2	2'-0"	6'-1"	2'-0"			
RA502	4		9'-4"	39		STR.						
RA503	16		10'-11"	182		3	3'-0"	2'-2"				
RA504	45		10'-9"	505		1	10"	10'-0"				
RA505	49	64	9'-2"	468	612	2	2'-8"	4'-1"	2'-8"			
RA506		68	10'-4"		733	1	10"	9'-5"				
RA507	60		8'-4"	522		2	2'-3"	4'-1"	2'-3"			
RA508	15		9'-9"	153		1	10"	9'-0"				
RA509	9		9'-5"	88		1	10"	8'-8"				
RA510	8		8'-11"	74		1	10"	8'-2"				
RA511	10		8'-3"	86		1	10"	7'-6"				
RA512	8		7'-9"	65		1	10"	7'-0"				
RA513	10		7'-2"	75		1	10"	6'-5"				
RA514	3		12'-10"	40		STR.						
RA515	72		30'-0"	2253		STR.						
RA516	20		12'-11"	269		1	2'-5"	10'-7"				
RA517	10		11'-10"	123		1	2'-2"	9'-9"				
RA518	5		8'-4"	43		1	2'-2"	6'-3"				
RA519	23		12'-0"	288		STR.						
RA520	2		13'-5"	28		STR.						
RA521		58	31'-7"		1911	STR.						
RA522	22		3'-11"	90		1	2'-0"	2'-0"				
RA523	16		6'-4"	106		STR.						
RA524	28		18'-8"	545		STR.						
RA525	4		11'-0"	46		STR.						
RA526	4		6'-3"	26		STR.						
RA527	2		10'-3"	21		19	8'-4"	1'-9 ⁵ / ₈ "	10 ¹ / ₈ "			
RA528	2		8'-3"	17		STR.						
RA529	2		11'-6"	24		STR.						
RA530	2		14'-8"	31		STR.						
RA531	6		16'-2"	101		STR.						
RA532	6		4'-6"	28		STR.						
RA533	8		10'-0"	83		STR.						
RA534	3		5'-6"	17		25	1'-8"	2'-5"	1'-5"	1 ¹ / ₂ "	5"	
RA535	5		5'-6"	29		STR.						
RA536	6		11'-11"	75		STR.						
RA537	6		7'-0"	44		STR.						
RA538	18		9'-3"	174		STR.						
RA539	14		7'-5"	108		23	1'-1"	3'-2"	3'-0"		3"	
RA601	45		18'-4"	1239		2	2'-7"	6'-1"	10'-0"			
RA602		32	18'-1"		869	2	2'-7"	6'-1"	9'-9"			
RA603	4	32	17'-9"	107	853	2	2'-7"	6'-1"	9'-5"			
RA604	15		17'-4"	391		2	2'-7"	6'-1"	9'-0"			
RA605	9		17'-0"	230		2	2'-7"	6'-1"	8'-8"			
RA606	8		16'-6"	198		2	2'-7"	6'-1"	8'-2"			
RA607	10		15'-10"	238		2	2'-7"	6'-1"	7'-6"			
RA608	8		15'-4"	184		2	2'-7"	6'-1"	7'-0"			
RA609	10		14'-9"	222		2	2'-7"	6'-1"	6'-5"			
RA610	10		18'-6"	278		2	8'-10"	1'-2"	8'-10"			
RA611	109	66	10'-7"	1733	1049	2	4'-9"	1'-5"	4'-9"			
RA612	65		12'-3"	1196		2	5'-7"	1'-5"	5'-7"			
RA613	109	66	6'-0"	982	595	7	2'-9"	11"	2'-0"	10"		
RA614	44	66	11'-7"	766	1148	2	5'-3"	1'-5"	5'-3"			
RA615	14	4	3'-10"	81	23	13	2'-0"	11"	2"	1'-1"		

BAR SCHEDULE

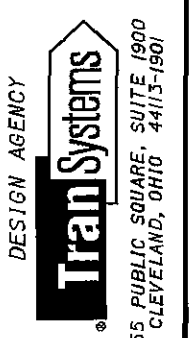
MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS					
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R
REAR ABUTMENT												
RA616	14	4	2'-11"	61	18	STR.						
RA617	1		4'-10"	7		STR.						
RA618	20		6'-7"	198		STR.						
	1 SERIES		13'-4"				6'-3"		6'-3"			
RA619	0F				118	2		1'-2"				6 ¹ / ₈ "
	8		6'-4"				2'-9"		2'-9"			
RA620	30		5'-7"	252		STR.						
RA621	11		25'-10"	427		2	12'-6"	1'-2"	12'-6"			
RA622	22		6'-9"	223		STR.						
RA623	1		9'-3"	14		STR.						
RA624	16		4'-3"	102		1	1'-7"	2'-10"				
RA625	24		11'-6"	412		31	2'-4"	2'-10"	1'-0"	1'-0"		
RA801	16		24'-9"	1057		32	24'-9"					
RA802	8		24'-9"	529		STR.						
RA803		16	34'-0"		1452	33	34'-0"					
RA804	16		25'-2"	1075		STR.						
RA805	4		13'-11"	149		STR.						
RA806	8		10'-5"	223		STR.						
RA807	4		7'-5"	79		STR.						
RA808	74	44	4'-11"	971	578	18	2'-7"	1'-0"	1'-0"			
RA901	16		4'-1"	222		STR.						
RA902	40		4'-2"	567		1	1'-7"	2'-10"				

TOTAL WEIGHT FOR REAR ABUTMENT = 33,011 LBS (PHASE I = 22,514 LBS, PHASE II = 10,497 LBS)

BAR BENDING DIAGRAMS



NOTE:
 FOR NOTES SEE SHEET 53/55.



DESIGNED	BTA	CHECKED	JDH
DRAWN	RCK	REVISED	JDH
REVIEWED	NFF	STRUCTURE FILE NUMBER	57087.10
DATE	05/31/05		

REINFORCING STEEL LIST
 BRIDGE NO. MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 IR-75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

Plotted By: emack Date: 7/27/2007 Time: 3:42:11PM
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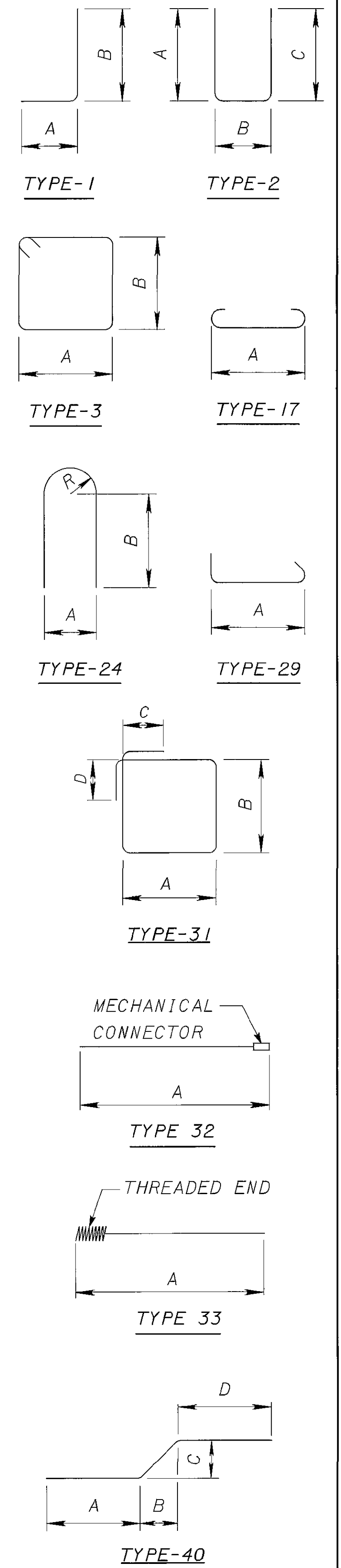
BAR SCHEDULE

MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS						
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R	INC.
PIER 1													
P401	4		14'-6"	39		3	2'-10"	4'-2"					
P402	944	624	3'-0"	1892	1250	29	2'-0"						
P501	64		22'-1"	1474		STR.							
P502		96	33'-0"		3304	STR.							
P503	64		29'-1"	1941		STR.							
P504	234	154	9'-0"	2197	1446	40	5'-2"	1'-2"	1'-2"	2'-3"			
P505	117	77	14'-3"	1739	1144	2	5'-2"	4'-2"	5'-2"				
P506	32		7'-1"	236		24	2'-0"	2'-0"			12"		
P507	32		28'-0"	935		STR.							
P508	32		21'-0"	701		STR.							
P601	24		11'-6"	412		31	2'-4"	2'-10"	1'-0"	1'-0"			
P602	16		4'-3"	102		1	1'-7"	2'-10"					
P603	8		4'-2"	50		STR.							
P801	8		14'-11"	319		STR.							
*P802		16	34'-0"		1452	33							
P803	8		29'-0"	619		STR.							
*P804	16		29'-2"	1246		32							
P805	16		6'-6"	278		STR.							
P901	40		4'-2"	567		1	1'-7"	2'-10"					
P1001	102	68	12'-4"	5413	3609	17	9'-6"						
P1002	234	154	11'-8"	11747	7731	1	1'-10"	10'-2"					
P1003	234	154	26'-9"	26935	17726	STR.							
TOTAL WEIGHT PIER 1 = 96,504 LBS (PHASE I = 58,842 LBS, PHASE II = 37,662 LBS)													
PIER 2													
P402	987	819	3'-0"	1978	1641	29	2'-0"						
P501	70		21'-1"	1539		STR.							
P502		112	33'-0"		3855	STR.							
P503	14		38'-6"	562		STR.							
P504	188	154	8'-1"	1585	1298	40	4'-2"	1'-2"	1'-2"	2'-3"			
P505	94	77	12'-4"	1209	991	2	4'-2"	4'-2"	4'-2"				
P506	42		7'-1"	310		24	2'-0"	2'-0"			12"		
P507	42		37'-5"	1639		STR.							
P508	42		20'-0"	876		STR.							
P601	24		11'-6"	412		31	2'-4"	2'-10"	1'-0"	1'-0"			
P602	16		4'-3"	102		1	1'-7"	2'-10"					
P603	8		4'-2"	50		STR.							
P801	8		21'-2"	452		STR.							
*P802		16	34'-0"		1452	33							
P803	8		19'-6"	417		STR.							
*P804	16		21'-0"	897		32							
P901	40		4'-2"	567		1	1'-7"	2'-10"					
P1001	72	61	12'-4"	3821	3237	17	9'-6"						
P1002	188	154	11'-8"	9438	7731	1	1'-10"	10'-2"					
P1003	188	154	35'-0"	28314	23193	STR.							
TOTAL WEIGHT PIER 2 = 97,566 LBS (PHASE I = 54,168 LBS, PHASE II = 43,398 LBS)													

BAR SCHEDULE

MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS						
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R	INC.
PIER 3													
P402	1012	858	3'-0"	2028	1719	29	2'-0"						
P501	14		38'-2"	557		STR.							
P502		116	33'-0"		3993	STR.							
P503	14		38'-6"	562		STR.							
P504	182	154	8'-1"	1534	1298	40	4'-2"	1'-2"	1'-2"	2'-3"			
P505	91	77	12'-4"	1171	991	2	4'-2"	4'-2"	4'-2"				
P506	44		7'-1"	325		24	2'-0"	2'-0"			12"		
P507	44		37'-5"	1717		STR.							
P508	44		37'-1"	1702		STR.							
P601	24		11'-6"	412		31	2'-4"	2'-10"	1'-0"	1'-0"			
P602	16		4'-3"	102		1	1'-7"	2'-10"					
P603	8		4'-2"	50		STR.							
P801	8		19'-2"	409		STR.							
*P802		16	34'-0"		1452	33							
P803	8		19'-6"	417		STR.							
*P804	16		21'-0"	897		32							
P901	40		4'-2"	567		1	1'-7"	2'-10"					
P1001	75	67	12'-4"	3980	3556	17	9'-6"						
P1002	182	154	11'-8"	9137	7731	1	1'-10"	10'-2"					
P1003	182	154	35'-2"	27541	23304	STR.							
TOTAL WEIGHT PIER 3 = 97,152 LBS (PHASE I = 53,108 LBS, PHASE II = 44,044 LBS)													
PIER 4													
P402	810	702	3'-0"	1623	1407	29	2'-0"						
P501	14		36'-2"	528		STR.							
P502		100	33'-0"		3442	STR.							
P503	14		38'-6"	562		STR.							
P504	178	154	8'-1"	1501	1298	40	4'-2"	1'-2"	1'-2"	2'-3"			
P505	89	77	12'-4"	1145	991	2	4'-2"	4'-2"	4'-2"				
P506	36		7'-1"	266		24	2'-0"	2'-0"			12"		
P507	36		37'-5"	1405		STR.							
P508	36		35'-1"	1317		STR.							
P601	24		11'-6"	412		31	2'-4"	2'-10"	1'-0"	1'-0"			
P602	16		4'-3"	102		1	1'-7"	2'-10"					
P603	8		4'-2"	50		STR.							
*P801	8		34'-2"	730		32							
*P802		16	34'-0"		1452	33							
P803	8		22'-3"	475		STR.							
*P804	8		18'-5"	393		32							
P901	40		4'-2"	567		1	1'-7"	2'-10"					
P1001	72	68	12'-4"	3821	3609	17	9'-6"						
P1002	178	154	11'-8"	8936	7731	1	1'-10"	10'-2"					
P1003	178	154	29'-7"	22659	19604	STR.							
TOTAL WEIGHT PIER 4 = 86,026 LBS (PHASE I = 46,492 LBS, PHASE II = 39,534 LBS)													

BAR BENDING DIAGRAMS



NOTES:

1. FOR NOTES, SEE SHEET 53/55.
 * SEE SHEET 53/55

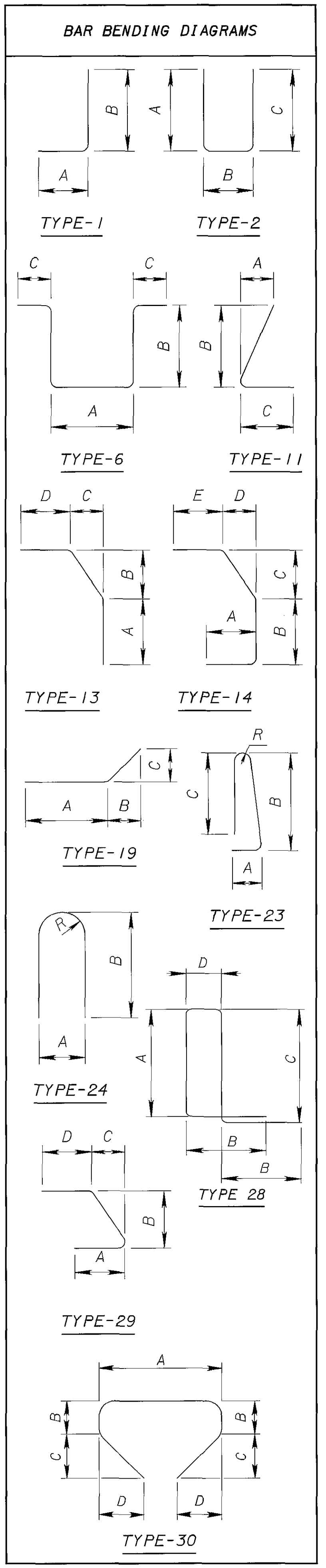
DESIGN AGENCY: **IranSystems**
 9615 W. 40th Ave. Suite 100
 Golden, CO 80401
 DATE: 05/30/05
 REVIEWED: RER
 STRUCTURE FILE NUMBER: 57087.10
 DRAWN: CAG
 CHECKED: NFF
 DESIGNED: JDH
 REINFORCING STEEL LIST
 BRIDGE NO.: MOT-75-1523 (PRESTRESSED CONCRETE OPTION)
 IR-75 OVER THE GREAT MIAMI RIVER
 MOT-75-14.60
 PID 23828
 54/55
 289
 314

Plotted By: ecmack Date: 7/27/2007 Time: 3:42:11 PM
 Filename: g:\coo04\0066\bridge\concrete_ait\cadd\23828r104.dgn

BAR SCHEDULE												
MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS					
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R
SUPERSTRUCTURE												
S401	2130	1640	30'-0"	42685	32866	STR						
S402	59		25'-6"	1005		STR						
S403	37		26'-4"	651		STR						
S404	24		22'-2"	355		STR						
S405	48	82	12'-6"	401	685	STR						
S406	34		20'-5"	464		STR						
S407	24		22'-3"	357		STR						
S408	258	208	8'-6"	1465	1181	2	3'-9"	1'-2"	3'-9"			
S409	38	32	4'-10"	123	103	30	1'-1 1/2"	6 1/4"	8 1/2"	8 1/2"		
S410	38	32	12'-11"	328	276	24	5 1/2"	6'-4"				2 1/2"
S411	38	32	2'-8"	68	57	STR						
S412	77		13'-7"	699		28	5'-10"	8"	5'-10"	11"		
S413	164	192	14'-10"	1625	1902	6	2'-2"	5'-10"	8"			
S501	4305	304	30'-0"	134703	9512	STR						
	2 SERIES		12'-10"									
S502	0F		T0	36720		STR						1/4"
	655		40'-11"									
S503	1448		40'-2"	60662		STR						
S504		5520	31'-7"		181837	STR						
	2 SERIES		8'-4"									
S505	0F		T0	40512		STR						1/16"
	1379		19'-10"									
S506	1		8'-4"	9		STR						
S507	16		4'-0"	67		STR						
S508	1181		7'-5"	9136		23	1'-1"	3'-2"	3'-0"			2 3/4"
S509	6		24'-7"	154		19	22'-7"	2'-0"	2 1/2"			
S510	6		21'-1"	132		STR						
S511		1138	5'-6"		6528	11	10 1/4"	4'-5"	1'-1"			
S512		1138	5'-5"		6429	1	1'-1"	4'-5"				
S513		8	11'-7"		97	STR						
S514		8	17'-5"		145	STR						
S515		8	14'-2"		118	STR						
S516		8	20'-0"		167	STR						
S517		16	8'-10"		147	STR						
S518	6		14'-4"	90		STR						
S519		8	5'-6"		46	13	2'-11"	1'-6 1/2"	3 1/2"	1'-1"		
S520		8	6'-3"		52	19	4'-5"	2 1/2"	1'-1"			
S521		8	2'-10"		24	STR						
S522	1		19'-10"	21		STR						
S523	1		40'-11"	43		STR						
S601		32	5'-0"		240	STR						
S602	43	40	30'-0"	1938	1802	STR						
S603	1181	1138	3'-9"	6652	6410	29	1'-1"	1'-9"	4"	1'-1"		
S604	1181	1138	2'-8"	4730	4558	1	1'-1"	1'-9"				
S605	1		22'-3"	33		19	17'-11"	4'-4"	5 1/2"			
S606	1		18'-9"	28		STR						
S607		1	29'-8"		45	STR						
S608		1	7'-2"		11	STR						
S609		2	18'-2"		55	STR						
S610	1		9'-8"	15		STR						
S611		2	2'-10"		9	STR						
S612		8	6'-3"		75	29	2'-7"	1'-9"	4"	2'-1"		
S613		8	6'-1"		73	2	2'-1"	1'-9"	2'-7"			
S614		1	37'-10"		57	STR						

BAR SCHEDULE												
MARK	NUMBER		LENGTH	WEIGHT		TYPE	DIMENSIONS					
	PHASE I	PHASE II		PHASE I	PHASE II		A	B	C	D	E	R
S615		1	15'-4"		23	STR						
S616	30		7'-9"	349		STR						
S617	189	288	6'-11"	1963	2992	STR						
S618	24		9'-6"	342		13	3'-10"	7"	7"	4'-10"		
S619	252	384	9'-1"	3438	5239	13	3'-10"	7"	7"	4'-5"		
S620	24	24	2'-1"	75	75	STR						
S621	8	8	10'-4"	124	124	14	4'-5"	1'-6"	7"	7"	3'-10"	
S622	18		6'-8"	180		STR						
S623	24		8'-11"	321		13	3'-10"	7"	7"	4'-3"		
S624	12		4'-9"	86		STR						
S625	28		8'-0"	336		13	3'-10"	7"	7"	3'-4"		
S626	20		8'-1"	243		13	3'-10"	7"	7"	3'-5"		
S627	15		5'-0"	113		STR						
S628	9		8'-11"	121		STR						
S629	12		7'-9"	140		13	3'-10"	7"	7"	3'-1"		
S630	9		4'-4"	59		STR						
S631	24		8'-6"	306		13	3'-10"	7"	7"	3'-10"		
S632	30		5'-8"	255		STR						
S633	9		4'-10"	65		STR						
S634	6		7'-8"	69		STR						
S635	16		9'-0"	216		13	3'-10"	7"	7"	4'-4"		
S636	12		6'-8"	120		STR						
S637	16		8'-5"	202		13	3'-10"	7"	7"	3'-9"		
S701	2038	1628	30'-0"	124970	99829	STR						
S702	49		36'-2"	3622		STR						
S703	31		39'-8"	2513		STR						
S704	22		26'-1"	1173		STR						
S705	42	74	15'-1"	1295	2281	STR						
S706	31		19'-0"	1204		STR						
S707	22		21'-6"	967		STR						
S708	741	438	30'-9"	46574	27530	STR						
S709	933	720	24'-1"	45928	35443	STR						
S801	12		10'-11"	350		13	3'-10"	7"	7"	6'-4"		
TOTAL WEIGHT SUPERSTRUCTURE = 1,012,633 LBS (PHASE I = 583,590 LBS, PHASE II = 429,043 LBS)												

- NOTES:**
- THE BAR SIZE NUMBER IS SPECIFIED ON THE PLANS IN THE BAR MARK COLUMN. THE FIRST DIGIT WHERE THREE DIGITS ARE USED, AND THE FIRST TWO DIGITS WHERE FOUR ARE USED, INDICATES THE BAR SIZE NUMBER. FOR EXAMPLE, RA601: RA: LOCATION OF THE BAR IN THE STRUCTURE (REAR ABUTMENT) 6: BAR SIZE DESIGNATION NO. 6 01: SEQUENCE NUMBER
 - BAR DIMENSIONS SHOWN ARE OUT TO OUT UNLESS OTHERWISE NOTED. "STD." WRITTEN IN PLACE OF A DIMENSION INDICATES A STANDARD BAR BEND AT THE END OF THE BAR. ALL REINFORCING STEEL IS TO BE EPOXY COATED. STRAIGHT BARS ARE INDICATED BY "STR".



DESIGN AGENCY
TransSystems
 50 CLEVELAND AVENUE SUITE 200
 CLEVELAND, OHIO 44115-4890

REVIEWED DATE 05/31/05
 NFF 5708710
 STRUCTURE FILE NUMBER

DRAWN MLR
 CHECKED JDH

REINFORCING STEEL LIST
 (PRESTRESSED CONCRETE OPTION)
 BRIDGE NO. MOT-75-1523
 I.R. 75 OVER THE GREAT MIAMI RIVER

MOT-75-14.60
 PID 23828

55/55
 290
 314