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PROPOSED WORK

THE PROPOSED WORK CONSISTS OF BUILDING A SYSTEM OF CAST-IN-PLACE RETAINING WALLS (WALL A, WALL B AND WALL D) AND INSTALLING SOIL NAILS AND A STRUCTURAL FACING (WALL C) ALONG THE EXISTING BIN WALL.

FROM STA (FEET)	TO STA (FEET)	WALL LENGTH (FEET)	WALL ID	NOTES	
11+61.17	11+61.17 11+74.79 13.62 N/A		N/A	BARRIER TRANSITION	
11+74.79 BEGIN WALL	12+25.00	50.21 N/A		TYPE D BARRIER	
12+25.00	14+86.00	261.00	WALL 'A'	GRAVITY WALL (WALL "A") -4.25' X 1.5' FOOTING -NO SHEAR KEY	
14+86.00	15+36.00	50.00	WALL 'B'	GRAVITY WALL (WALL "B") -4.25' X 1.5' FOOTING, PLUS -2' X 1.5' SHEAR KEY	
15+36.00	18+98.00	362.00	WALL 'C'	SOIL NAIL WALL (WALL "C") -2 ROWS FROM STA 15+35 TO STA 15+87.5 -3 ROWS FROM STA 15+87.5 TO STA 18+98	
18+98.00	19+17.34 END WALL	19.34	WALL 'D'	GRAVITY WALL (WALL "D") -5' X 1.5' FOOTING, PLUS -2' X 1.5' SHEAR KEY	

DESIGN SPECIFICATIONS:

THIS STRUCTURE CONFORMS TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION, 2017, AND THE ODOT BRIDGE DESIGN MANUAL, 2007, INCLUDING REVISIONS THROUGH JANUARY 2018, AND SPECIAL PROVISIONS FOR THE DEL-23-11.71 PID 98141 PROJECT, SECTION 900 FOR PERMANENT SOIL NAILS AND SECTION 1000 FOR SHOTCRETE FACING.

CAST IN PLACE GRAVITY WALLS GENERAL NOTES

THE FOLLOWING NOTES PERTAIN TO THE THE CAST IN PLACE GRAVITY WALLS IDENTIFIED AS WALL 'A', WALL 'B' AND WALL 'D'.

THE WORK CONSISTS OF FURNISHING AND INSTALLING PERMANENT CAST IN PLACE GRAVITY WALLS AS SHOWN IN THE PLANS.

UTILITIES THAT MAY BE AFFECTED ARE TO BE FIELD LOCATED AND MOVED IF NECESSARY PRIOR TO BEGINNING OF GRAVITY WALLS CONSTRUCTION.

OVERALL CONSTRUCTION SEQUENCE IN SHEET 10/128 SHALL BE FOLLOWED FOR THE CONSTRUCTION OF THE PROPOSED GRAVITY WALLS.

THE DEPARTMENT WILL PAY FOR GRAVITY WALLS AT THE CONTRACT UNIT PRICE FOR EACH OF THE FOLLOWING PAY ITEMS.

ITEM	UNIT	DESCRIPTION
509	LB	EPOXY COATED REINFORCING STEEL
511	CY	CLASS QC1 CONCRETE WITH QC/QA
		RETAINING/WINGWALL NOT INCLUDING
		FOOTING
511	CY	CLASS QC1 CONCRETE, FOOTING

DESIGN PARAMETERS:

FOUNDATION SOIL	
UNIT WEIGHT OF SOIL	125 PCF
ANGLE OF FRICTION	O DEG
EFFECTIVE ANGLE OF FRICTION	26 DEG
EFFECTIVE COHESION	250 PSF
UNDRAINED SHEAR STRENGTH	3000 PSF

SURCHARGE SOIL	
UNIT WEIGHT OF SOIL	120 PCF
ANGLE OF FRICTION	O DEG
EFFECTIVE ANGLE OF FRICTION	24 DEG
EFFECTIVE COHESION	125 PSF
UNDRAINED SHEAR STRENGTH	1250 PSF

DESIGN DATA:

CONCRETE CLASS QC1 COMPRESSIVE STRENGTH 4000 PSI

REINFORCING STEEL ASTM A615 OR A996 GRADE 60, MINIMUM YIELD STRENGTH 60 KSI

FOUNDATION BEARING PRESSURE GRAVITY WALL FOOTINGS, AS DESIGNED, PRODUCE THE FOLLOWING BEARING PRESSURE:

FOUNDATION BEARING PRESSURE GRAVITY WALL FOOTINGS, AS DESIGNED, PRODUCE THE FOLLOWING BEARING PRESSURE:

Wall ID	SERVICE BEARING PRESSURE (KSF)	STRENGTH BEARING PRESSURE (KSF)	BEARING RESISTANCE (KSF)	
WALL "A", GRAVITY	0.86	1.35	5.95	
WALL "B", GRAVITY	1.05	1.59	6.05	
WALL "D", GRAVITY	1.28	1.39	6.41	

CAST IN PLACE FOOTINGS

THE CONTRACTOR SHOULD CUT THE FOOTING AND FOOTING KEYWAY TO NEAT EXCAVATION LINES WITHOUT FORMING PRIOR TO PLACING THE FOOTING CONCRETE AND REINFORCING STEEL. FORM THE FOOTING ONLY WHEN THE EXCAVATION WALLS ARE UNSTABLE.

EXISTING BIN WALL SURACE PREPARATIONS

THE WORK SHALL INCLUDE ANY PREPARATORY TRIMMING AND CLEANING OF SOIL ROCK AND CLEANING THE SHOTCRETE RECEIVING SURFACES FOR THE SOIL NAIL RETAINING WALL.

CLEAN THE FACE OF THE EXISTING BIN WALL AND OTHER SURFACES TO BE SHOTCRETED OF LOOSE MATERIALS, MUD, REBOUND, OVERSPRAY, OR OTHER FOREIGN MATTER THAT COULD PREVENT OR REDUCE SHOTCRETE BOND. PROTECT ADJACENT SURFACES FROM OVERSPRAY DURING SHOOTING. AVOID LOOSENING, CRACKING, OR SHATTERING THE GROUND DURING EXCAVATION AND CLEANING. REMOVE ANY SURFACE MATERIAL WHICH IS SO LOOSENED OR DAMAGED, TO A SUFFICIENT DEPTH TO PROVIDE A BASE THAT IS SUITABLE TO RECEIVE THE SHOTCRETE. REMOVE MATERIAL THAT LOOSENS AS THE SHOTCRETE IS APPLIED. ALL MATERIALS, LABOR AND INCIDENTALS NECESSARY TO COMPLETE THE ABOVE DETAILED WORK SHALL BE INCIDENTAL TO "ITEM 690 - SPECIAL - MISC.: SHOTCRETE FACING TO FILL EXISTING BIN WALL CORRUGATION, INCLUSIVE."

SOIL NAIL WALL GENERAL NOTES:

ITEM 690 SOIL NAIL, 15 KIP DESIGN TEST LOAD ITEM 690 PROOF TEST ON SOIL NAILS ITEM 690 VERIFICATION TEST ON SOIL NAILS

THE WORK CONSISTS OF FURNISHING, AND INSTALLING, AND PROOF AND VERIFICATION TESTING SOIL NAILS FOR PERMANENT SOIL NAIL WALLS AS SHOWN IN THE PLANS IN ACCORDANCE WITH THE SPECIAL PROVISION "900 PERMANENT SOIL NAILING".

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THE DEPARTMENT WILL PAY FOR SOIL NAILS AT THE CONTRACT UNIT PRICE PER EACH PRODUCTION SOIL NAIL WITH 15 KIP MAXIMUM TEST LOAD.

HOLLOW BAR SOIL NAILS (HBSN) ARE ALLOWED. FOR HBSN, MAKE MODIFICATIONS TO THE INSTALLATION PROCEDURE, MATERIALS, AND GROUTING PER FHWA-NHI-14-007 GEC 7 - SOIL NAIL WALLS REFERENCE MANUAL (2015) CHAPTER 10, SUBJECT TO ACCEPTANCE BY THE ENGINEER. HBSN SHALL HAVE THE SAME OR GREATER INITIAL CROSS-SECTIONAL AREA TO A #8 THREADED BAR.

A MINIMUM OF 3 SOIL NAILS SHALL BE SUBJECTED TO VERIFICATION TESTS AS PER SECTION 900 FOR PERMANENT SOIL NAILS. AFTER TESTING, VERIFICATION TEST NAILS SHALL BE CUT OFF AT THE EXISTING WALL FACE AND ABANDONED.

A MINIMUM OF 11 SOIL NAILS SHALL BE SUBJECTED TO PROOF TESTS AS PER SECTION 900 FOR PERMANENT SOIL NAILS.

ANY SOIL NAIL SHALL ONLY BE SUBJECTED TO EITHER VERIFICATION TEST OR PROOF TESTS BUT NOT BOTH.

THE DEPARTMENT WILL PAY FOR EACH SOIL NAIL VERIFICATION TESTED AT THE CONTRACT UNIT PRICE OF "ITEM 690 SPECIAL MISC.: VERIFICATION TEST NAILS" AND WILL PAY FOR EACH SOIL NAIL PROOF TESTED AT THE CONTRACT UNIT PRICE OF "ITEM 690 SPECIAL MISC.: PROOF TEST NAILS"

UTILITIES THAT MAY BE AFFECTED BY SOIL NAIL SYSTEM ARE TO BE FIELD LOCATED AND MOVED IF NECESSARY PRIOR TO BEGINNING OF SOIL NAIL CONSTRUCTION.

MINIMUM SOIL NAIL DRILLED LENGTHS (L) SHALL BE 20 FEET INSTALLED INCLINED AT 15 DEGREES FROM THE HORIZONTAL.

OVERALL CONSTRUCTION SEQUENCE IN SHEET 12/128 SHALL BE FOLLOWED FOR THE CONSTRUCTION OF THE PROPOSED SOIL NAIL WALLS.

DUE TO THE PRESENCE OF EXISTING PILES FROM STRUCTURE DEL-23-11.89 (US23 OVER US36 & DELAWARE RUN), THE SPACING OR DRILLING ANGLE OF THE SOIL NAILS MAY NEED TO BE ADJUSTED IN THE FIELD TO AVOID DRILLING THROUGH THE EXISTING PILES. THE APPROXIMATE LOCATIONS OF THE TOP OF THE EXISTING PILES ARE

- 1. STATION *18+89, 31' LT.
- 2. STATION *18+96, 33' LT.
- 3. STATION *18+99, 31' LT.

THESE PILES WERE LOCATED USING THE 1964 ORIGINAL CONSTRUCTION PLANS DEL-23-10.23. THE CONTRACTOR SHALL USE THE AS BUILT LOCATIONS OF THE TOP OF THE EXISTING PILES TO DEVELOP OR ADJUST THE SHOP DRAWINGS FOR THE SOIL NAILS.

SOIL NAIL WALL GENERAL NOTES (CONTINUED):

IN THE CASE THAT DRILLING FOR A SOIL NAIL HOLE HITS AN EXISTING PILE, THE DRILLED HOLE SHALL BE ABANDONED. THE HOLE SHALL BE FILLED WITH GROUT AND THE SOIL NAIL HOLE SHALL BE RELOCATED WITHIN I FOOT OF THE ORIGINAL SOIL NAIL LOCATION. THE DEPARTMENT WILL PAY FOR EACH ABANDONED SOIL NAIL AT 25 PERCENT OF THE PRODUCTION SOIL NAIL UNIT PRICE. THE PAYMENT FOR THE ABANDONED SOIL NAILS INCLUDES THE COST OF DRILLING, ABANDONING THE HOLE, AND GROUTING INCLUDING: TIME, MATERIAL, AND LABOR. FOR ESTIMATING PURPOSES THE CONTRACTOR SHALL ASSUME THAT THE WALL WILL HAVE 2 EXCAVATED SOIL NAILS THAT WILL NEED TO BE ABANDONED AND RELOCATED.

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DESIGN DATA

PARAMETER	SPECIFICATION
DRILL HOLE DIAMETER	6 INCHES
MINIMUM NAIL LENGTH	20 FEET
DESIGN TEST LOAD (DTL)	15 KIPS
MAXIMUM NAIL SPACING	
HORIZONTAL	5 FEET
VERTICAL *	5 FEET
NAIL INCLINATION	15° FROM HORIZONTAL
NAIL BAR	
MINIMUM SIZE	#8 THREADED BAR
MINIMUM GRADE	75 KSI
NAIL GROUT COMPRESSIVE STRENGTH	3,000 PSI 28-DAY 1,500 PSI 3-DAY
REINFORCING STEEL	WELDED WIRE FABRIC COATED GRADE 60 MIN. YIELD (EPOXY COATED) STRENGTH 60 OR 75 KSI, ODO CMS 509.00

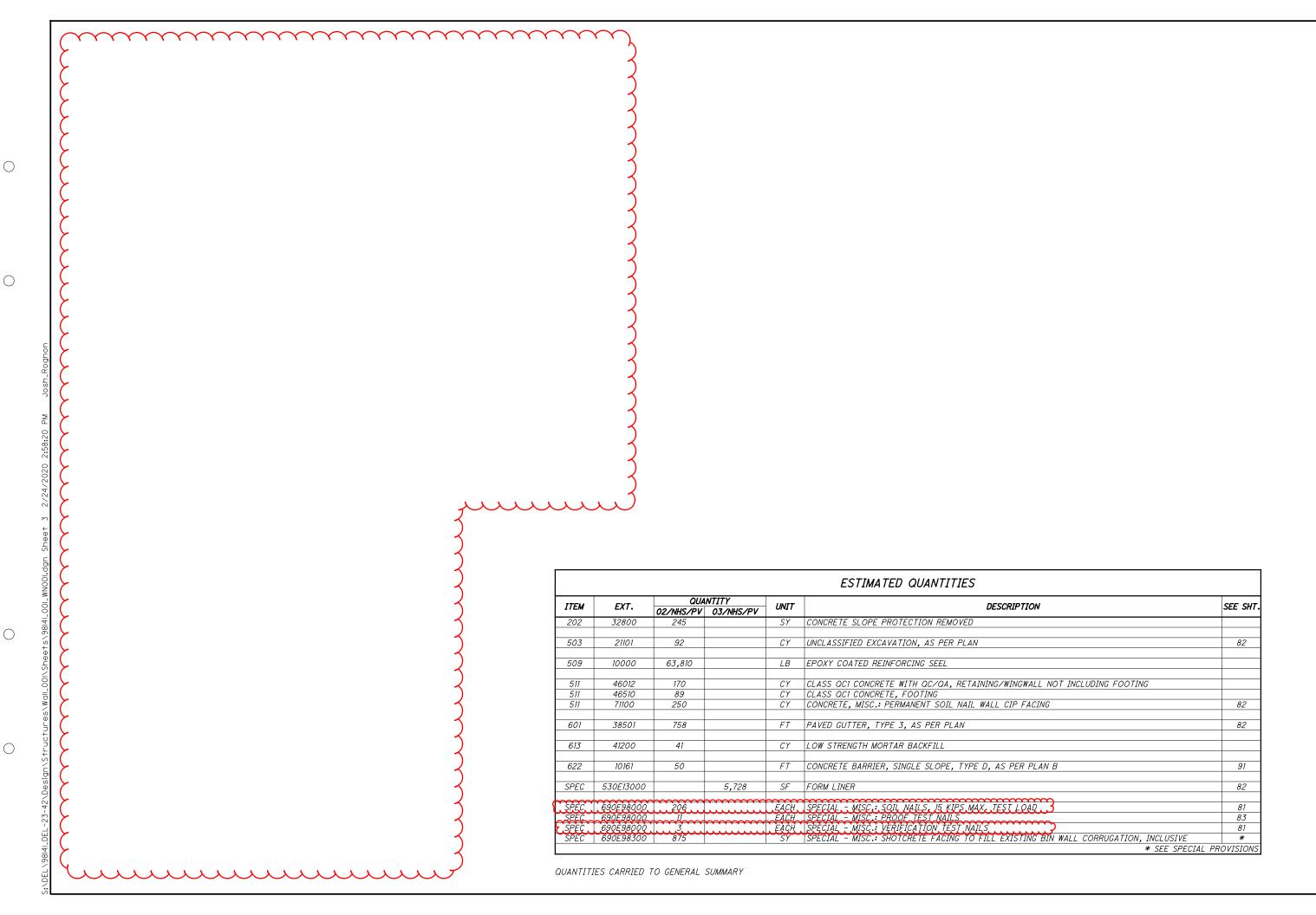
* FROM STA. 15+87 TO STA. 16+87 BOTTOM ROW VERTICAL NAIL SPACING (SV) IS 4 FEET.

STRUCTURE DRAINAGE, 4" DIAMETER PVC PIPE WEEPHOLE

THE WORK CONSISTS OF FURNISHING AND INSTALLING 4 INCH DIAMETER PVC PIPE WEEPHOLE AS DETAILED IN SHEETS 91-91 FOR THE GRAVITY AND SOIL NAIL RETAINING WALLS. THE DRAIN PIPE WILL OUTLET DIRECTLY TO ITEM 518 - POROUS BACKFILL UNDERLAIN BY ITEM 512 - TYPE 3 WATERPROOFING MEMBRANE. THE TYPE 3 WATERPROOFING SHALL BE APPLIED TO THE ENTIRE TOP SURFACE OF THE FOOTING AND SHALL EXTEND VERTICALLY DOWN THE ENTIRE FOOTING SIDES. ITEM 518 - POROUS BACKFILL WILL OUTLET TO THE EXISTING UNDERDRAIN EXCEPT AT WALL "D" (GRAVITY WALL), LOCATION WHERE THE EXISTING UNDERDRAIN IS TO BE REPLACED WITH A NEW UNDERDRAIN AT WALL "C" (SOIL NAIL WALL) AS SOON AS PRACTICAL. THE APPROXIMATE LOCATION OF THE EXISTING UNDERDRAIN IS SHOWN IN THE DETAIL.

CARE SHALL BE TAKEN DURING THE PLACEMENT OF THE PVC PIPE NOT TO ENTRAP DIRTOR EXCESSIVE DUST THAT COULD CAUSE CLOGGING OF THE DRAINAGE SYSTEM. PVC DRAINAGE PIPE SHALL BE IN ACCORDANCE WITH ITME 707.45 AND TYPE 3 WATERPROOFING SHALL BE IN ACCORDANCE WITH ITEM 711.29.

ALL MATERIALS, LABOR AND INCIDENTALS NECESSARY TO CONSTRUCT THE 4" DIA. PVC PIPE WEEPHOLE INCLUDING ITEM 518 - POROUS BACKFILL AND ITEM 512 - TYPE 3 WATERPROOFING SHALL BE INCIDENTAL TO THE SHOTCRETE FACING PAY ITEM 690 - SPECIAL - MISC.: SHOTCRETE FACING TO FILL EXISTING BIN WALL CORRUGATION, INCLUSIVE.



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