Project:	Bridge No. FRA-70-1395C	Design:	RFV
Subject:	Estimated Quantities - FINAL	Check:	DJC
Date:	########		

ITEM 202 - STRUCTURE REMOVED, OVER 20 FOOT SPAN

area =	11616.79	sf
unit cost =	\$18.00	per sf

Lump sum = <u>\$209,102</u>

ITEM 202 - APPROACH SLAB REMOVED

(+ Sidewalk	Curves) =	<u>336</u> sy
		Total =	334 sv
width =	60	ft	
length =	25	ft	

ITEM 202 - WEARING COURSE REMOVED

length = 190.52 ft width = 60 ft

Total =	1271	sv
10tul =		~,

ITEM 503 - COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN

ITEM 503 - UNCLASSIFIED EXCAVATION

Rear Abutn	nent:		
length =	156.94	ft	Note:
width =	27	ft	Excavation for abutments will
depth =	20.75	ft	be taken up when the exist.
			abutments are removed
Forward Ak	outment:		
length =	151.77	ft	
width =	22	ft	
depth =	28.5	ft	
Abutment	Subtotal =		6781 cy
Piers:			
length =		148	ft
width =		17	ft
depth =		11	ft
no. of ftgs p	oer pier =	1	
no. of piers	=	1	
	_		
Pier Subto	tal =	1025	су

Total = <u>7806</u> cy

ITEM 509 - EPOXY COATED REINFORCING STEEL

Totals below are from the reinforcing steel lists in the plans

6			\sim	
(Slab (parapets) =	17,953 lbs	く	Rev.
(Slab (bridge) =	185,742 lbs	2	
7	Slab (east cap) =	98,649 lbs)	
7	Sidewalk =	10,564 lbs	1	
(Superstructure subtotal =	312,908 lbs	く	
(
7	Rear Abutment =	25,027 lbs	- ⁻)	
5	Frwd. Abutment =	23,630 lbs	<u> </u>	
(Abutment subtotal =	48,657 lbs	く	
(
7	Pier & Footing =	<mark>110,481</mark> lbs)	
4	Pier subtotal =	110,481 lbs	く	
(ノ	
(Approach slabs =	112,668 lbs)	
7	Approach slab subtotal =	112,668 lbs	1	
5			く	
(Total = $\frac{584,714}{100}$ lbs		
9		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	\sim	

ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK

BRIDGE

DCOR				
thickness =	9.25	in		
edge of deck				
to bridge limits =	1.25	ft		
total sum of spans =	196.25	ft		
total length =	198.82			
O/O of deck width =	103.25	ft		
Deck Volume =	586	су		
Haunch:				
t/flange width =	18	in		
t/deck to t/web =	13	in		
haunch thick. =	2.5	in		
t/flange thick. (avg) =	1.25	in		
total no. of beams =	10			
Haunch Volume (Inte	rior Beams	s) =		32 cy
Haunch Volume (Inte Haunch Volume (Exte	erior Beams erior Beam	s) = s) =		32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Ext	erior Beams erior Beam	s) = s) =		32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever:	rior Beams erior Beam	s) = s) =		32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exter Cantilever: cantilever length (right	rior Beams erior Beam	s) = s) =	2.88 ft	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever: cantilever length (right cantilever length (left)	rior Beams erior Beam) = =	s) = s) =	2.88 ft 2.88 ft	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever: cantilever length (right cantilever length (left) Cantilever Volume =	rior Beams erior Beam) = =	s) = s) =	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Ext Cantilever: cantilever length (right cantilever length (left) Cantilever Volume =	rior Beams erior Beam) = =	s) = s) =	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag	rior Beams erior Beam) = = JMS (Bridg e	s) = s) = e):	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag length =	rior Beams erior Beam) = = jms (Bridge 10.83	s) = s) = s) = ft	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Exte Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag length = thickness =	rior Beams erior Beam) = = 10.83 3.00	s) = s) = e): ft	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Ext Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag length = thickness = height =	rior Beams erior Beam) = = 10.83 3.00 2.80	s) = s) = e): ft ft ft	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Ext Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag length = thickness = height = number =	rior Beams erior Beam) = = 10.83 3.00 2.80 3.00 3.00	<pre>>) = s) = >): ft ft ft each</pre>	2.88 ft 2.88 ft 10 cy	32 cy 6 cy
Haunch Volume (Inte Haunch Volume (Ext Cantilever: cantilever length (right cantilever length (left) Cantilever Volume = Signal Pole Diaphrag length = thickness = height = number = Diaph. Volume =	rior Beams erior Beam) = = 10.83 3.00 2.80 3.00 10	<pre>>) = >) = >) = >): ft ft ft each cy</pre>	2.88 ft 2.88 ft 10 cy	32 cy 6 cy

BRIDGE TOTAL = 644 cy

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CAP					
Deck:					
area =	7941.214	sf			
avg. thickness =	8.5	in (avg.)			
beam length =	198.75	ft			
Deck volume =	208	су			
Haunch:					
t/flange width =	18	in			
t/deck to t/web =	12.25	in			
haunch thick. $(avg) =$	2.4375	in			
t/flange thick. (avg) =	1.3125	in			
total no. of beams =	5				
haunch length =	198.75	ft			
Haunch Volume (Inter	or Beams)	=		12	CV
Haunch Volume (Exter	rior Beams)	=		6	cy
	,				-
Cantilever:					
cantilever length (right)) =	2	2.00 ft		
cantilever length (left)	=	2	2.00 ft		
Cantilever Volume =			6 cy		
Signal Pole Diaphrag	ms (East C	ap):			
length =	9.00	ft			
thickness =	3.00	ft			
height =	2.80	ft			
number =	1.00	each			
Diaph. Volume =	3	су			
CAP TOTAL =	217	су			
	Total =		<u>861</u> cy		
ITEM 511 - CLASS QC2 CONCR	ETE WITH	QC/QA,	BRIDGE I	DEC	K (PARAPET), AS PER PLAN

area (west) = 4.42 sf area (east) = 4.42 sf length (west) = 132.60 ft length (east) = 144.89 ft west pylon (rear end) = 248.55 cf west pylon (frwd. end) = 209.34 cf east pylon (rear end) = 231.00 cf east pylon (frwd. end) = 220.17 cf

<u>80</u> су

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS

Total =

	Total =	<u>393</u> су
pedestals =	32.32	cf
window thickness =	2.00	ft
window area =	436.90	sf
thickness =	3.00	ft
wall area =	3238.52	sf

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING, AS PER PLAN

Skew angle =	12.97	degrees	
Rear Abutment: Beam Seat beam seat area (above beam seat width = lower beam seat area = lower beam seat width Beam Seat Volume =	e paneling) = =	634.99 6.75 156.94 5.50 5149.34	sf (cad) ft sf (cad) ft <i>cf</i>
Backwall backwall area = backwall thickness = approach slab thickness abutment length = Back Wall Volume =	:S =	682.99 1.75 1.25 156.94 1097.14	sf (cad) ft ft ft <i>cf</i>
Additonal Volume pedestals = end wall volumes = Rear Abutment Subto	otal	16.92 191.79 239	cf cf cy
Beam Seat beam seat area (above beam seat width = lower beam seat area = lower beam seat width	e paneling) =	527.19 6.75 151.77	sf (cad) ft
Beam Seat Volume =	=	5.50 4393.27	ft <i>cf</i>
Beam Seat Volume = Backwall backwall area = backwall thickness = approach slab thickness abutment length = Back Wall Volume =	= :S =	5.50 4393.27 673.50 1.75 1.25 151.77 1083.77	ft cf sf (cad) ft ft ft cf
Beam Seat Volume = Backwall backwall area = backwall thickness = approach slab thickness abutment length = Back Wall Volume = Additonal Volume pedestals = end wall volumes = Forward Abutment Se	= :s = ubtotal =	5.50 4393.27 673.50 1.75 1.25 151.77 1083.77 16.92 105.26 207	ft cf sf (cad) ft ft ft cf cf cy

ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, FOOTING, AS PER PLAN

Pier:		
height =	3	ft
width =	15	ft
length =	147.66	ft

Panel Footing:

area =	5.33	sf
length =	308.71	ft

Total = <u>308</u> cy

ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, SIDEWALK, AS PER PLAN

Sidewalk on BRIDGE:

Sidewalk Volume =	130	су
avg. thickness =	8.50	in
area =	4957.46	sf

Sidewalk on CAP:

Sidewalk Volume =	134	су
avg. thickness =	10.56	in
cadd area =	4106.78	sf

Total = <u>264</u> cy

ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)

Rear Abutment:	Face	2967.00	sf
	Тор	863.17	sf
Forward Abutment :	Face	4546.72	sf
	Тор	1012.00	sf
Pier :	Face	2649.34	sf

Total = <u>1,338</u> sy

ITEM 512 - TYPE 2 WATERPROOFING

Rear Abutment = 9.68 ft Forward Abutment = 8.47 ft

Total = 7 sy

ITEM 512 - SEALING OF CONCRETE SURFACES (NON-EPOXY)

west naranet -	906 90	ef	
west parapet =	500.50	31	
east parapet =	1155.07	sf	
pylons =	451.87	sf	
west sidewalk =	2906.80	sf (includes small area of s	sidewalk on SW abutment)
east sidewalk =	2388.40	sf	
east cap =	3539.28	sf ====> cap total =	394 sy

Total = <u>1261</u> sy

ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 4

BRIDGE	
Girders:	
Section 1	
length =	65.50 ft
weight =	225.43 lbs/ft
Section 2	
volume =	43.53 cf
unit weight steel =	21327.4 lbs
Section 3	
length =	67.25 ft
weight =	225.43 lbs/ft
Detail factor =	1.0
No. of beams =	10
Beam Subtotal =	512,600 lbs

Splices: Top Flange	# of plates	length (in)	width (in)	thick (in)
outside plates –	1	37	18	0.625
insido platos -	2	37	8	0.625
fill plates =	2	10 5	10	0.025
nii piales = Bottom Elange	I	18.5	18	0.75
outside plates =	1	44	18	0.75
inside plates –	2	44	8	0.75
fill plates =	1	22	18	0.5
Web	•			0.0
plates =	2	29	22.5	0.5
Plate weight/splice =	853	lbs		
Splice Bolts	# of bolts	length (in)	bolt wt.	washer wt
Top Flange =	40	1.5	148	11.3
Bot. Flange =	48	1.5	148	11.3
Web =	48	1.5	148	11.3
	* from stee	l manual		
	* washer w	eight is per 1	00 ct.	
Bolt + Washer weight				
Top Flange =	64	lbs		
Bot. Flange =	76	lbs		
Web =	76	lbs		
Total weight/splice =	1070	lbs		
No. of splices =	20			
Splice + Bolts Subto	tal =	21,395	lbs	
Intermediate Crossfr	ames:			
Length of Diagonals =	10.67	ft (weighted	avg.)	
NO. OF Diagonals =	10.00	ft (eischte d	-	
Length of Horiz. =	10.62	it (weighted	avg.)	
NO. OI HOIZ. =		lbo/ft		
Angle weight / it. =	9.00	105/11		
Crossframe weight =	313	lbs ==> per	x-frame ass	sembly
x-frame stiffeners?	V	v or n		
Length =	34.000	in ==> web o	depth	
Width =	5.000	in		
Thickness =	0.375	in		
Stiffener weight =	36	lbs ==> per	x-frame ass	sembly
Total Intermediate Cro	ssframe			
Assembly Weight =	349	lbs. ==> per	x-frame as	sembly
No. of assemblies =	143			,
End Crossframes:				
Length of Discovered	10.05	ft (weighted		
Length of Diagonals =	10.85	it (weighted	avg.)	
IND. OF DIAGONAIS =	2	ft (mainten 1	-	
Length of Horiz. =	10.78	it (weighted	avg.)	
NO. OT HOMZ. =	1	lbc/ft		
Angle weight / It. =	9.80	IDS/IL		
Crossframe weight –	318	lhs> ner	v-frame as	combly

x-frame stiffeners?	УУ	/ or n		
Length =	5.000 i	n ==> web depth		
Width =	34.000 i	n		
Thickness =	0.375 i	n		
Stiffener weight =	36 I	bs ==> per x-frame as	sembly	
Total x-frame End				
Assembly Weight =	354 I	bs ==> per x-frame as	sembly	
No. of assemblies =	18			
Crossframe Subtotal = 56,339 lbs				
Embedded steel plate	(20"x20"x1.7	75") = 198	lbs	
Support angles (L6x4)	Support angles $(L6x4x1/2)$ - Length =			
Support ar	igle weight pe	er ft = 16.2	lb/ft	
No. of sup	oort angles =	- 2	ea	
No. of signal support of	liaphragms =	- 3	ea	
Signal support subtotal = 1,648 I			lbs	
BRIDGE STRUCTUR	AL STEEL =	591,983	lbs	

CAP

Section 1 length = weight =	65.50 202.47	ft Ibs/ft		
Section 2 web cadd area = web volume = top flange volume = bot. flange volume = volume = unit weight steel = Section 3	243.79 12.70 14.44 16.56 43.70 21411.72	sf cf cf cf cf lbs		
length =	67.25	ft Ibo/ft		
weight =	202.47	105/11		
Detail factor =	1.0			
No. of beams =	5			
Beam subtotal =	241,500	lbs		
Beam subtotal = Splices: Top Flange	241,500 # of plates	lbs length (in)	width (in)	thick (in)
Beam subtotal = Splices: <i>Top Flange</i> outside plates = inside plates = fill plates =	241,500 # of plates 1 2 1	lbs length (in) 37 37 18.5	width (in) 18 8 18	thick (in) 0.625 0.625 0.875
Beam subtotal = Splices: <i>Top Flange</i> outside plates = inside plates = fill plates = <i>Bottom Flange</i> outside plates = inside plates = fill plates = <i>Mob</i>	241,500 # of plates 1 2 1 1 2 1	Ibs length (in) 37 37 18.5 44 44 44 22	width (in) 18 8 18 18 18 18 18 18 18 18 18 18 18 1	thick (in) 0.625 0.625 0.875 0.75 0.75 0.75
Beam subtotal = Splices: <i>Top Flange</i> outside plates = inside plates = fill plates = <i>Bottom Flange</i> outside plates = inside plates = fill plates = <i>Web</i> plates =	241,500 # of plates 1 2 1 2 1 2 1 2	Ibs length (in) 37 37 18.5 44 44 22 29	width (in) 18 8 18 18 18 8 18 22.5	thick (in) 0.625 0.625 0.875 0.75 0.75 0.75 0.75

Splices Bolts Top Flange = Bot. Flange = Web =	# of bolts 40 48 48 * from stee * washer w	length (in) 1.5 1.5 1.5 I manual reight is per 1	bolt wt. 148 148 148 148	washer wt. 11.3 11.3 11.3
Bolt + Washer weight Top Flange = Bot. Flange = Web = Total weight/splice =	64 76 76 1110	lbs lbs lbs		
No. of splices =	11 006	lbo> plot	oo i holta	
Splice Subtotal =	11,090	105 ==> piat		
Intermediate Crossfr	ames:			
Length of Diagonals = No. of Diagonals =	8.91 2	ft (weighted a	avg.)	
Length of Horiz. = No. of Horiz. =	8.8 <mark>2</mark> 1	ft (weighted a	avg.)	
Angle weight / ft. =	9.80	lbs/ft		
Crossframe weight =	261	lbs ==> per >	-frame as	sembly
x-frame stiffeners? Length = Width = Thickness =	y 34.000 5.000 0.375	y or n in ==> web c in in	lepth	
Stiffener weight =	36	lbs ==> per >	c-frame as	sembly
Total x-frame assembly weight = No. of assemblies =	297 64	lbs ==> per >	<pre>c-frame as</pre>	sembly
End Crossframes:				
Length of Diagonals = No. of Diagonals =	9.00 2	ft (weighted a	avg.)	
Length of Horiz. =	8.90	ft (weighted a	avg.)	
Angle weight / ft. =	9.80	lbs/ft		
Crossframe weight =	264	lbs ==> per >	-frame as	sembly
x-frame stiffeners? Length = Width = Thickness =	y 34.000 5.000 0.375	y or n in ==> web d in in	lepth	
Stiffener weight =	36	lbs ==> per >	-frame as	sembly
Total x-frame				
assembly weight = No. of assemblies =	300 8	IDS ==> per >	(-trame as	sembly
Crossframe Subtota	=	21,418	lbs	

Signal Support Diaphragm:		
Embedded steel plate (20"x20"x1.75") =	198	lbs
Support angles (L6x4x1/2) - Length =	10.000	ft
Support angle weight per ft =	16.2	lb/ft
No. of support angles =	2	ea
No. of signal support diaphragms =	1	ea
Signal support subtotal =	522	lbs
CAP STRUCTURAL STEEL = 274,536	lbs	

Total = <u>866,519</u> lbs

ITEM 513 - WELDED STUD SHEAR CONNECTORS

Bridge	No. rows per beam = No. per row = No. of beams =	259 3 10	
	Number per signal sup	port =	50
	Number of signal supp	orts =	3
	Bridge Total =	7920	ea
Cap	No. rows per beam =	321	
	No. per row =	3	
	No. of beams =	5	
	Number per signal sup	port =	50
	Number of signal supp	1	
	Cap Total =	4865	ea

Total = <u>12,785</u> ea

ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, FINISH COAT

Bridge	web area =		1208.53	sf
	section 1 flange area =		315.22	sf
	section 2 flange area =		334.06	sf
	section 3 flange area =		323.64	sf
	No. of beams =		10	
	Crossframes =		6873.23	sf
	Stiffeners =		817.30	sf
	Utility Supports =		1089.18	sf
	Bridge Subtotal =		30,594	sf
Cap	web area =		1208.53	sf
	section 1 flange area =		311.13	sf
	section 2 flange area =		332.67	sf
	section 3 flange area =		319.44	sf
	No. of beams =		5	
	Crossframes =		2559.79	sf
	Stiffeners =		365.50	sf
	Cap Subtotal =		13,784	sf
		Total =	<u>44,378</u>	sf

ITEM 514 - FINAL INSPECTION REPAIR

Length =	198.75	ft
No. Girders =	15	ea
No. Crossframes =	233	ea

Total = <u>32</u> ea

ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL, AS PER PLAN (4")

RA SW Corner =	5.04	ft	(Bridge)
RA Length =	106.38	ft	(Bridge)
RA Length =	41.47	ft	(Cap)
RA SE Corner =	23.08	ft	(Cap)
RA Backwall - 4W1 =	4.92	ft	(Cap)
Bridge - Cap =	198.93	ft	(Cap)
FA Length =	106.05	ft	(Bridge)
FA Length =	41.47	ft	(Cap)
FA NE Corner =	19.84	ft	(Cap)
FA Backwall - 4W2 =	2.96	ft	(Cap)
			,
Brid	lge Total =		<u>218</u> ft
C	ap Total =		<u>333</u> ft
	Total =		551 ft

ITEM 516 - 1" PREFORMED EXPANSION JOINT FILLER

Rear Abutment:		
beam seat height =	5.33	ft
beam seat width =	6.75	ft
beam seat length =	156.94	ft
wall height =	17.65	ft
wall width =	0.92	ft
backwall height =	4.31	ft
backwall width =	1.75	ft
Forward Abutment:		
beam seat height =	4.02	ft
beam seat width =	6.75	ft
beam seat length =	151.77	ft
wall height =	20.55	ft
wall width =	0.92	ft
backwall height =	4.44	ft
backwall width =	1.75	ft
	Total =	<u>397</u> sf
ITEM 516 - 2" PREFORMED EXP	PANSION J	IOINT FILLER

Pier: height = 27.85 ft Total =

<u>84</u> sf

ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES & LOAD PLATE (NEOPRENE) 10.5" x 1'-4" x 2.45" PAD WITH 11.5" x 1'-10" BEVELED PLATE, AS PER PLAN

<u>30</u> ea

Rear Abutment =	15	ea
Forward Abutment =	15	ea

Total =

ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES & LOAD PLATE (NEOPRENE) 1'-6" x 2'-0" x 3.40" PAD WITH 1'-7" x 2'-9" BEVELED PLATE, AS PER PLAN

Total = <u>15</u> ea

ITEM 518 -SCUPPER, MISC .:

Total = 2 ea

ITEM 518 - POROUS BACKFILL WITH GEOTEXTILE FABRIC

avg. top of backwall = top of drilled shaft = height = length =	753.73 744.45 7.53 ft 156.94 ft	
thickness =	2.00 ft	
area (panel footing) =	6.58 sf	
Rear Abutment Subto	3,397 cf	
Forward Abutment:		
avg. top of backwall =	757.48	
top of drilled shaft =	748.35	
height =	7.38 ft	
length =	151.77 ft	
thickness =	2.00 ft	
area (panel footing) =	6.58 sf	
Forward Abutment S	ubtotal =	3,238 cf
	Total =	<u>246</u> cy

ITEM 518 - 6" PERFORATED CORRUGATED PLASTIC PIPE

ent:	
313.88 ft	(inlcudes façade panel pipe)
	ent: 313.88 ft

Forward Abutment: length = 303.54 ft

(inlcudes façade panel pipe)

Total = 620 ft

ITEM 518 - 6" NON-PERFORATED CORRUGATED PLASTIC PIPE

Rear Abutment:	00 (i	
length =	20 ft	
Forward Abutment:		
length =	27 ft	
	Total =	<u>50</u> ft
ITEM 518 - STRUCTURE DRAIN	AGE, MISC.:	
ITEM 518 - STRUCTURE DRAIN	AGE, MISC.: 45.25 ft	
ITEM 518 - STRUCTURE DRAIN length 1 = length 2 =	45.25 ft 53.75 ft	
ITEM 518 - STRUCTURE DRAIN length 1 = length 2 = drop =	AGE, MISC.: 45.25 ft 53.75 ft 2.5 ft	
ITEM 518 - STRUCTURE DRAIN length 1 = length 2 = drop =	45.25 ft 53.75 ft 2.5 ft	

ITEM 524 - DRILLED SHAFTS, 60" DIAMETER, ABOVE BEDROCK WITH QC/QA, AS PER PLAN

Rear Abutment:

length =	54.45 ft
no. of columns =	31

Forward Abutment:

length = 58.35 ft no. of columns = 45

Total = 4,314 ft

ITEM 526 - REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=15"), AS PER PLAN

Rear Approach Slab: area = 3957.64 sf

Forward Approach Slab: area = 4656.49 sf

Total = <u>958</u> sy

ITEM SPECIAL - STRUCTURE, MISC.: PERMANENT UTILITY SUPPORTS

Length of Horiz. = No. of Horiz. =	10.78 ft (weighted avg.)
Angle weight / ft. =	12.80 lbs/ft
utility support weight =	138 lbs ==> per assembly
Stiffeners?	y y or n
Length =	5.000 in ==> web depth
Width =	34.000 in
Thickness =	0.375 in
Stiffener weight =	36 lbs ==> per x-frame assembly
Total utility support	
assembly weight =	174 lbs ==> per x-frame assembly
No. of assemblies =	56
Total Weight =	9752 lbs

ITEM SPECIAL - STRUCTURE, MISC.: PRECAST FAÇADE PANELS

Rear Abutment: area = 2769.95 sf

Forward Abutment: area = 3261.59 sf

Total = 6.032 sf

ITEM 607 - FENCE, MISC .: WALL MOUNTED TYPE A (W/ VANDAL MESH)

Rear Abutment:length =4.5 ft ==> on SW knee wall extension

North end of west parapet:

length = 15.50 ft ==> north of NW end pilaster

Total = 20 ft

ITEM 625 - LIGHT POLE ANCHOR BOLTS, MISC.: COMBINATION SIGNAL POLE AND PEDESTRIAN POLE ANCHOR BOLT ASSEMBLIES EMBEDDED IN CONCRETE BRIDGE DECK

