

Geotechnical Evaluation
GB 1 Subgrade Analysis

I-70 South Trench

W-07-109

Prepared For:

MS Consultants, Inc.
2221 Schrock Road
Columbus, Ohio 43229

January 2010

**GEOTECHNICAL EVALUATION
GB 1 SUBGRADE ANALYSIS**

**I-70 SOUTH TRENCH
W-07-109**

PREPARED FOR:

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JANUARY 2010

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1.0 INTRODUCTION

The Ohio Department of Transportation (ODOT) proposes to reconstruct the east and south portions of the Interstate (I) I-70/I-71/I-670 Innerbelt located in downtown Columbus, Ohio. This system of highways, bridges, and interchanges extends in a south to north direction along I-71 and its convergence with State Route (SR) 315 from Greenlawn Avenue to West Broad Street and also in a south to north direction from the I-70/I-71 split north along I-71 to the I-670 interchange. The limits of the project extend along I-70 in a west to east direction from a point near Sullivant Avenue to Fairwood Avenue. Reconstruction of the entire system includes the replacement or reconstruction of I-70 East and I-70 West as well as I-71 North and I-71 South within the project area. Replacement or construction of bridges, abutments, and roadways will be included.

The project area for this GB 1 subgrade analysis report is defined as the I-70 South Trench located between the I-70/SR 315 split (West Interchange) and I-70/I-71 East Interchange. The project site location is shown on **Figure 1**, contained in **Appendix A** of this report (all figures are contained in **Appendix A**). The area generally extends in a west to east direction from the railroad overpass east of Whittier Street to Grant Avenue. Subgrade improvements for other segments of the project area will be submitted in separate GB 1 subgrade analysis reports. The proposed mainline I-70 and associated ramps are included. The locations of the proposed South Trench mainline roadways and ramps are shown on **Figure 2**. Soil borings used to analyze subgrade soils and conditions for the area were drilled by DLZ Ohio, Inc. (DLZ). Subsurface investigations were performed in accordance with ODOT's "Specifications for Geotechnical Explorations," dated January 16, 2009, applicable Geotechnical Bulletins distributed by ODOT Office of Geotechnical Engineering, and applicable American Society for Testing and Materials (ASTM) guidelines. Information from borings drilled for retaining walls was also used to analyze subgrade soils for pavement design if the borings were located adjacent to the proposed roadway alignments.

ODOT Geotechnical Bulletin GB 1, titled "Plan Subgrades," dated January 18, 2007, was used to analyze the soils information gathered during the subsurface investigation. Information from the soil boring logs and laboratory analysis results of the soil samples collected from the borings was entered into the GB 1 data analysis spreadsheet provided by ODOT. Results of the GB 1 analysis were used to prepare the recommended subgrade stabilization methods and extent of subgrade repairs that are summarized in the last

section of this document. The following sections provide a general description of the subsurface conditions in the vicinity of the South Trench, GB 1 analysis input parameters, GB 1 analysis results, and subgrade improvement recommendations.

2.0 SUBSURFACE INVESTIGATION

2.1 Subsurface Investigation Methods

Soil borings were drilled by DLZ with either a truck-mounted or all terrain vehicle (ATV)-mounted drill rig. The soil borings that provide relevant subsurface information for the South Trench are located on **Figure 2** through **Figure 6**. **Table 1**, located in **Appendix B**, is a listing of the soil borings used for this section of the project area. Borings are listed in order of the associated segment of mainline roadway or ramp. In addition to the borings drilled within the proposed pavement area, borings drilled for retaining wall design at various locations within the South Trench were used for analysis. If these borings provided pertinent subsurface information and they were near the proposed edge of pavement, they were included in the analysis.

During drilling, field personnel made measurements and observations of subsurface conditions and recorded them on boring logs. Boring logs for the segment of the project discussed in this report are contained in **Appendix E**. After drilling, each boring location was surveyed in the field for location and elevation. The boring locations (northing and easting) were provided by DLZ in Ohio State Plane coordinates. Elevations were provided by DLZ in North American Datum of 1983.

The borings were drilled using rotary-type drilling methods. At regular intervals, disturbed, but representative soil samples, were obtained using a 2-inch outside diameter (OD) split-barrel sampler driven into the soil by blows from a 140-pound hammer free falling 30 inches (Standard Penetration Test [SPT] - ASTM D1586). Driving resistance was recorded in the field and listed on the boring logs in terms of the number of hammer blows for each 6-inch driving interval. The second and third intervals (N_2 and N_3) are entered into the GB 1 analysis spreadsheets and are added together to calculate the number of blows per foot (N). GB 1 analysis also requires the entry of drill rod energy ratio (ER) for each boring. The drill rod energy is expressed as a percent and is associated with N in the following equation:

$$N_{60} = N_m \times (ER/60)$$

where: N_m = the measured N value ($N_2 + N_3$)
ER = drill rod energy ratio as a percent for the specific rig used
 N_{60} = 60% energy ratio

Three different drill rigs were used to obtain soil samples for the project area. DLZ provided the various ERs for each boring installed. The ERs for the equipment used in this project are listed below:

Notation on GB 1 Analysis	Drill Rod Energy Ratio
A	61
B	62
C	63

2.2 General Geologic Material

The State of Ohio Department of Natural Resources (ODNR), Division of Geological Survey Bulletin 44 indicates that the general area of the site was glaciated by both the Illinoian- and Wisconsin-age ice sheets, and that the unconsolidated deposits are relatively thick. According to bedrock topography maps of the area, the top of bedrock varies from Elevation 600 to Elevation 650 within the South Trench. This would suggest the depth to bedrock is greater than 75 feet below the existing site grades and would not affect pavement subgrade design.

There are no significant drainage features located within the South Trench area. The topography of the site varies significantly north to south throughout this area as a result of the existing I-70 roadway. The original surface grades in this area were generally between Elevations 750 and 770 prior to construction of the existing I-70 roadway and South Trench. The construction of this depressed roadway created a trench-type condition for portions of the South Trench that are approximately 20 to 30 feet below the surrounding grades.

2.3 General Subsurface Conditions Encountered in the Borings

The following paragraphs provide a general description of the subsurface conditions encountered in the soil borings. Locations of the borings used for the subgrade analysis are shown on the **Figure 2**. For a detailed description of the conditions encountered at each boring location, refer to each individual boring log, contained in **Appendix E**.

Soil borings located within the South Trench area encountered competent glacial till soils or dense granular outwash material. In general, the test borings penetrated approximately 10 to 136 feet below existing grades. Surface materials in the area included topsoil, concrete pavement, or gravel. **Table 2** is a summary of the topsoil, pavement, and pavement base material thicknesses encountered at each boring location in the project area as reported on the boring logs (boring logs are included in **Appendix E**). Surface material thicknesses were noted in order to establish the sampling interval depths of subgrade soils to be input into the GB 1 spreadsheet analysis since topsoil, pavement, and aggregate base materials are not applicable to the analysis.

Soils information used for the pavement subgrade analysis was limited to the first 6 feet of soil below proposed subgrade elevations by the GB 1 analysis spreadsheet. Primarily fine grained (generally cohesive) soils were encountered near surface. The SPT blow counts were generally high and correspond to relative densities of dense or better for granular soils, and for fine grained soils the consistency would typically be correlated as stiff to very stiff. Bedrock was not encountered within the first 6 feet of finished subgrade in any of the borings. The moisture content of the soil samples exhibiting cohesive properties was typically -6 to +6 points from optimum; with optimum moisture content based on the soil classification rather than site-specific laboratory analysis.

2.4 Groundwater Observations

The elevation of groundwater observed in boreholes during drilling was noted on each boring log. Water level information is included in **Table 2**. Observed water elevations in the borings ranged from 9.2 to 43.3 feet below ground surface (bgs). Several borings were reported as "dry" to depths up to 20 feet.

The true groundwater levels cannot be discerned from the information provided. Short-term water level readings in boreholes, especially in cohesive soils, are an indication, but not an accurate measurement of the groundwater elevation in an area. The long-term groundwater elevations would need to be confirmed by installing monitoring wells or piezometers and performing long-term (seasonal) groundwater measurements.

2.5 Laboratory Testing

Laboratory testing on recovered soil samples consisted of ODOT soil classification, moisture content, Atterberg limits, and grain size analysis. Tests results are reported on the boring logs contained in **Appendix E** and within the GB 1 analysis data discussed in Section 3.0.

3.0 PRELIMINARY PAVEMENT SUBGRADE EVALUATION AND RECOMMENDATIONS

3.1 GB 1 Analysis Data Input

The project area for this report is defined as the immediate area of the proposed South Trench for I-70. The proposed mainline I-70 and ramps were included. Table 1 included in Appendix B shows the soil borings used for the subgrade analysis for each segment evaluated. The complete project area was analyzed on one GB 1 analysis spreadsheet in addition to each segment of the project being analyzed on separate GB 1 analysis spreadsheets.

A GB 1 analysis required data input into ODOT provided spreadsheets that will generate recommended subgrade stabilization measures, where necessary. Input data required from each soil boring are as follows:

- Boring number
- Boring location
- Sample depths (from proposed subgrade elevation)
- N_2 and N_3 (blow counts for second and third 6-inch intervals)
- drill rig ER
- liquid limit and plastic limit
- percent silt and percent clay
- moisture content
- soil classification.

The GB 1 analysis spreadsheet calculates the following information for each boring location:

- N_m – number of blows per foot required to drive the sample tube through the soil.
 $N_M = N_2 + N_3$, sum of blow counts for the second and third 6-inch increments for each split spoon sample drive
- N_L – the lowest N_{60} value recorded in the top 6 feet of material

- $N_{60} - N_M$ corrected to an equivalent drill rod energy of 60 percent
- PI – Plasticity Index
- P_{200} – percent of material passing No. 200 sieve
- M_{opt} – optimum moisture content (based on soil type)
- Problems with subgrade soil relative to the soil classification
- Problems with subgrade soil relative to the soil moisture or soil strength estimated from blow counts
- Possible subgrade treatments (undercutting, cement stabilization [CS], or lime stabilization [LS]).

Sample depths presented on the boring logs were changed to depth below design subgrade prior to input into the GB 1 analysis spreadsheet. This will establish the depth of undercutting or depth of stabilization needed from design subgrade rather than from a reference elevation of the top of each boring (existing grade at the time of drilling). Soil sample results for soils that will be cut out prior to establishing the design subgrade elevations were not entered into the GB 1 analysis.

3.2 GB 1 Analysis Results – Entire I-70 South Trench Area

As required by GB-1, the following is a tabulation of the average results for the entire I-70 South Trench project area:

Average NL for entire South Trench area	22.0
Average PI to the nearest whole number	10
Average design California Bearing Ratio (CBR) to the nearest whole number (as an average not a percent)	9
Average moisture content to the nearest whole number	10

The GB 1 analysis spreadsheet output for the entire South Trench project area is contained in **Appendix C**. The project area was also broken up into segments to complete individual analyses for each roadway segment and ramp. The GB 1 analysis output for each individual segment is contained in **Appendix D**. **Table 3** presents the GB 1 analysis results of the individual segments. The calculated design CBRs ranged from 8 to 11 as indicated in **Table 3**.

The designer, based on the results of subsurface exploration, is responsible for identifying the method, location, and dimensions (including depth) of subgrade stabilization in the plans. GB 1 is to be used as general guidance. Limits for subgrade stabilization estimated in this GB 1 analysis should be verified and adjusted in the field based on proof rolling and visual observation. Undercutting estimates and treatment options were based on the following parameters:

- Soils at a depth of 6 feet or greater below finished subgrade were not considered in the subgrade improvements to be implemented since the soils were too deep.
- Maximum undercut depth was set at 6 feet from design subgrade since the undercut will be filled with competent material.
- Where the GB 1 analysis indicated an undercut depth, the undercut depth was applied only to the sample interval if the underlying soils were indicated to not need undercutting by the GB 1 analysis.
- Where the GB 1 analysis indicated the top sample in the boring required undercutting, the undercut was started at the top of the sample interval needing cut.

The soils classifications data for the project area identified in the boring program were identified as type A-1a, A-1b, A-2-4, A-3, A-3a, A-4a, A-4b, A-6a, A-6b, and A-7-6. Out of the identified materials, only soil type A-4b will be removed or stabilized based on soil type alone.

The moisture content of cohesive soils has a significant effect on the physical properties of the soil. Moisture content reported on the boring logs only represents the moisture content during drilling and sampling. Actual moisture content during construction may vary

greatly from the measured soil moisture content reported. Undercut areas required by GB 1 analysis presented in this report are based on soil type and excessive moisture in the samples collected. The extent and need for subgrade improvement is dependent on the actual subgrade conditions during site preparation and construction. Each area of subgrade for proposed pavements must be confirmed in the field by proof rolling to identify the need for subgrade stabilization.

The following sections summarize the results of the GB 1 analyses performed for individual roadway segments or ramps. As described below, no subgrade stabilization methods are evidenced based on the results of the GB 1 output for the South Trench.

3.3 GB 1 Analysis I-70 Eastbound

Eleven soil borings were used to determine subgrade stabilization needs for I-70 eastbound reconstruction. The calculated design CBR for this portion of the project area is 9.

No areas for undercutting or subgrade stabilization were identified. Boring B-031 on **Figure 5** shows the presence of A-4b soils located at depths greater than 3 feet below proposed subgrade and therefore these soils need not be removed.

3.4 GB 1 Analysis I-70 Westbound

Eleven soil borings were used to determine subgrade stabilization needs for the proposed I-70 westbound reconstruction in the project area. The calculated design CBR for this portion of the project area is 9. No areas for undercutting or subgrade stabilization were identified.

3.5 GB 1 Analysis Ramp F1

Ramp F1 is located south of the I-70 mainline on the west end of the South Trench. The majority of Ramp F1 is bridge section. One section of Ramp F1, approximately from Station 264+50 to Station 267+50, is not a bridge section. Boring B-018 was used to determine subgrade stabilization needs for the proposed Ramp F1. Ramp F1 is shown on **Figures 3 and 4**. The final design alignment and grade for Ramp F1 shows approximately 9.7 feet of fill material will be added to the ramp subgrade at boring B-018. Since the

amount of fill is greater than 8 feet, the GB 1 analysis spread sheet did not calculate an average group index or a design CBR for the ramp segment. According to the GB 1 analysis, undercutting or other stabilization methods are not required for the 300-foot section of Ramp F1 in the project area.

3.6 GB 1 Analysis Ramp M1

One soil boring (B-022) was used to determine subgrade stabilization needs for the proposed Ramp M1 in the project area. Ramp M1 is shown on Figures 3 and 4. The calculated design CBR for this portion of the project area is 11. According to the GB 1 analysis, undercutting or other subgrade stabilization methods are not required for Ramp M1. Only the eastern portion of Ramp M1 is pavement construction from approximately Station 625+50 to 631+00. The western portion of Ramp M1 is a bridge section.

3.7 GB 1 Analysis Ramp N1

One soil boring (B-043) was used to determine subgrade stabilization needs for the proposed Ramp N1 in the project area. Ramp N1 is shown on Figure 6. The calculated design CBR for this portion of the project area is 8. According to the GB 1 analysis, undercutting or other subgrade stabilization methods are not required for the portion of Ramp N1 located west of Grant Avenue. The GB 1 analysis for the portion of Ramp N1 located east of Grant Avenue is contained in a separate GB 1 analysis document for the Southeast Interchange.

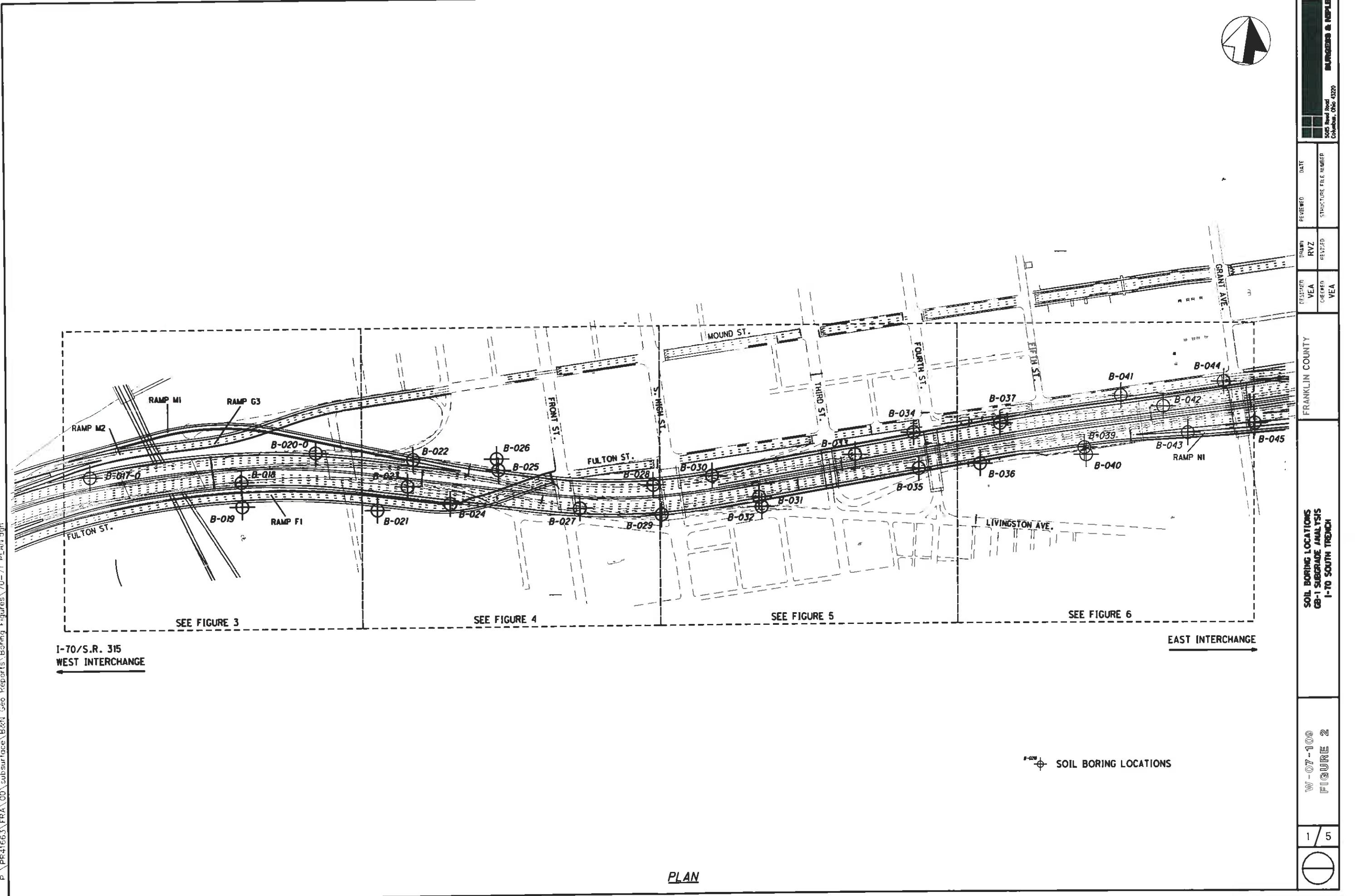
3.8 Pavement Subgrade Recommendations

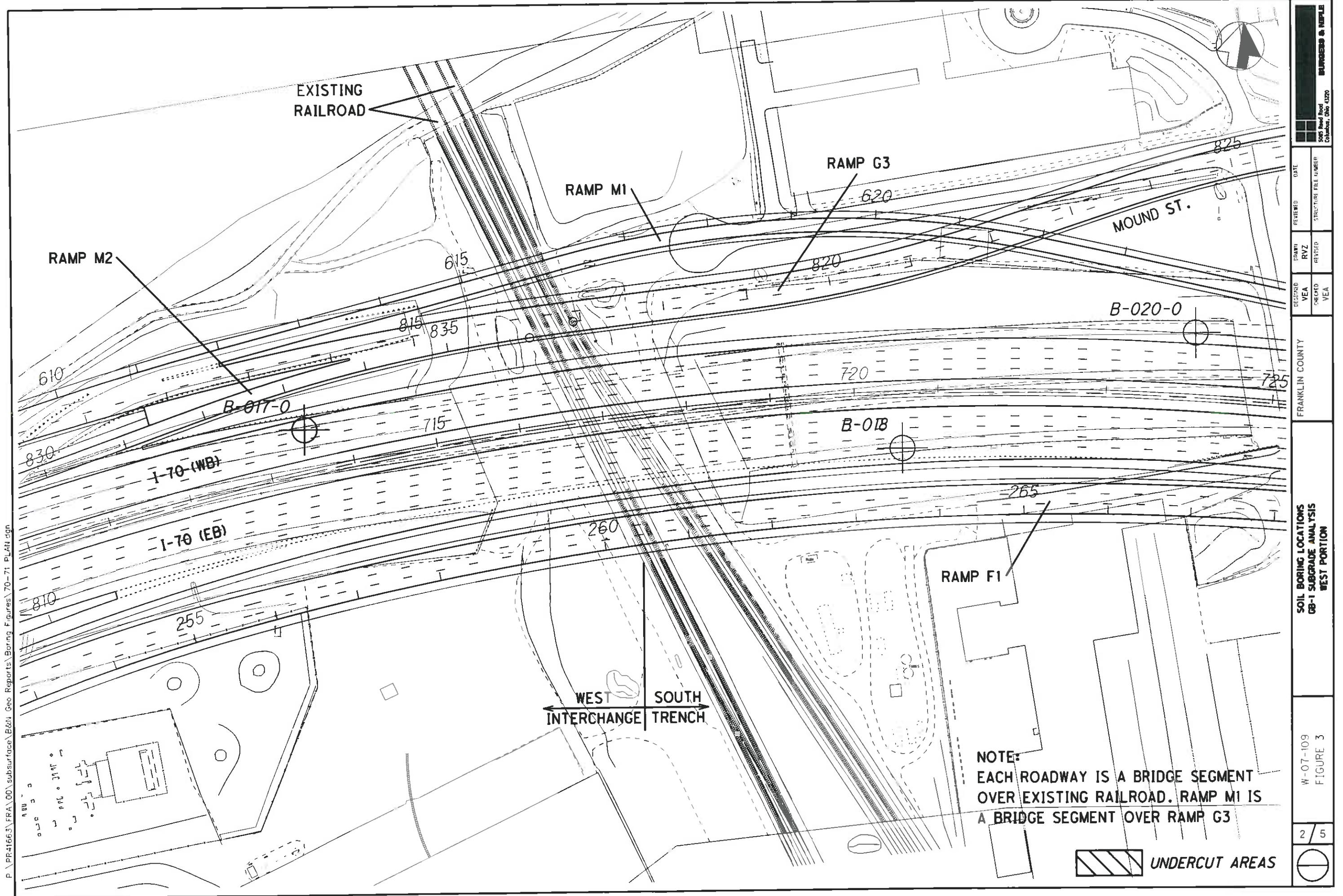
All recommendations for subgrade stabilization improvement/treatments for the South Trench area are based on the Subgrade Version 9.09 as provided by ODOT and described in GB 1. Undercutting is not required in the South Trench project area (22 boring locations). Cement stabilization is an option in one area of westbound I-70 (boring B-034) but is not considered practical due to the limited area needing cement stabilization. Lime stabilization was not listed as an option in any areas of the South Trench project site.

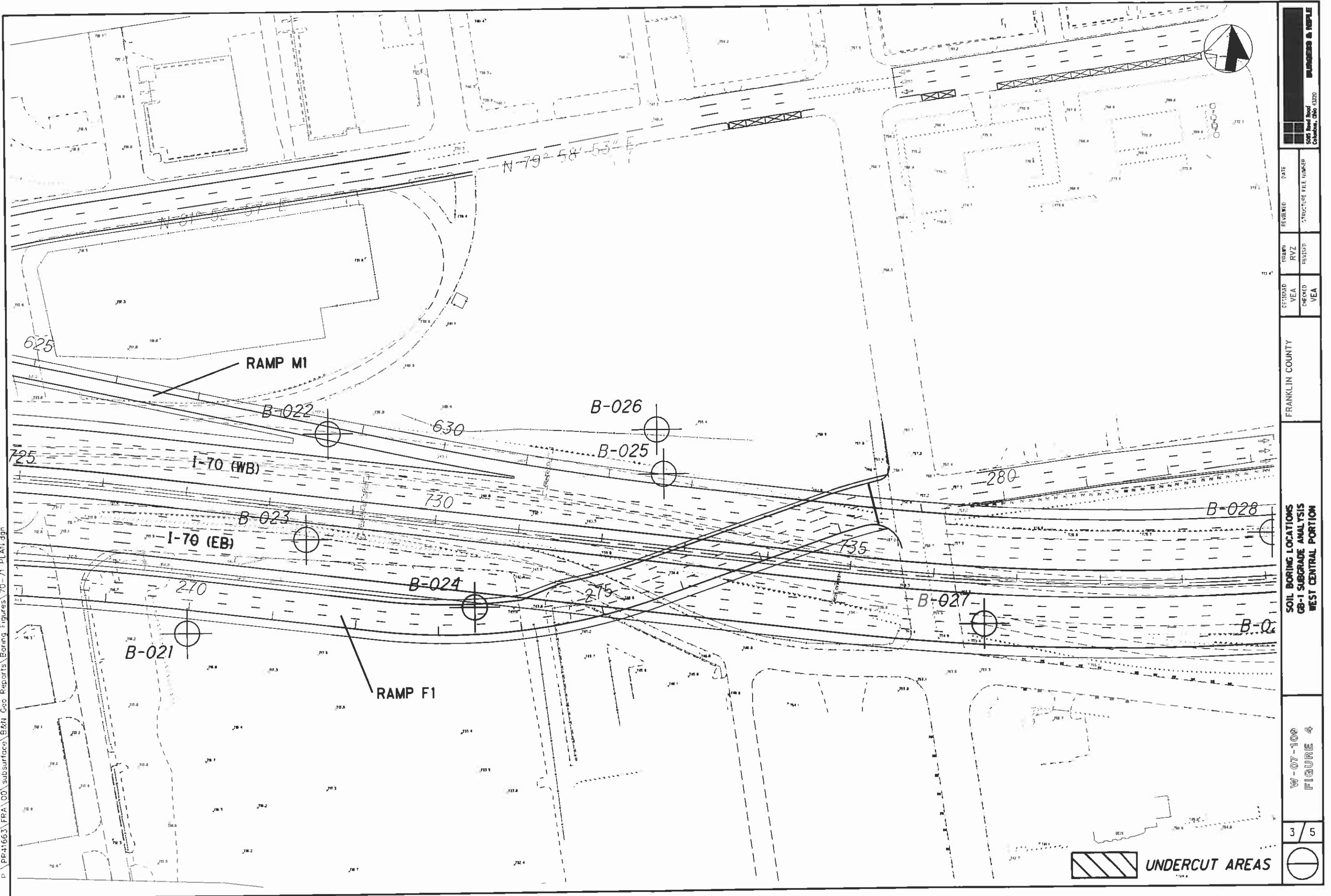
APPENDIX A

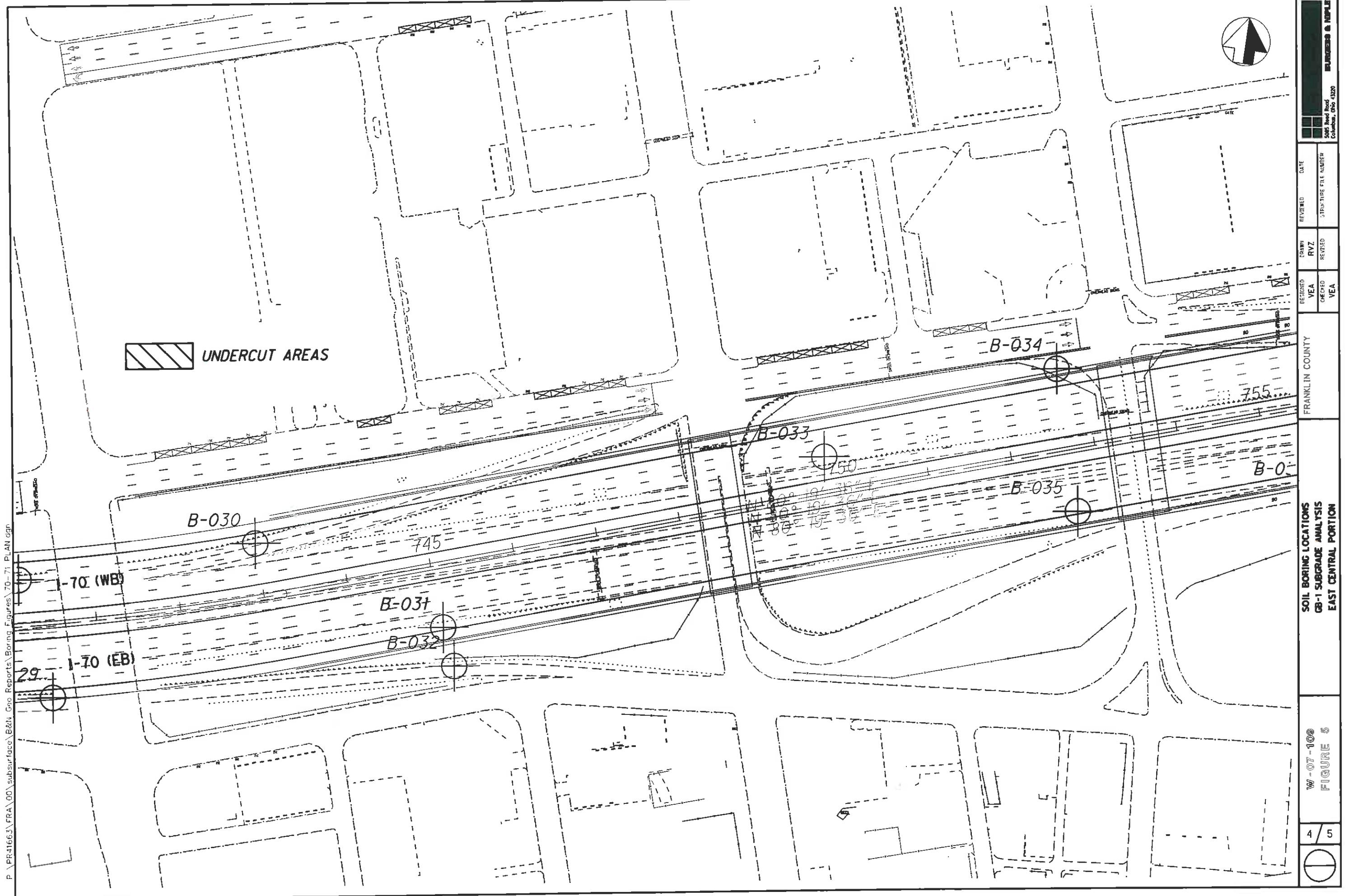
FIGURES

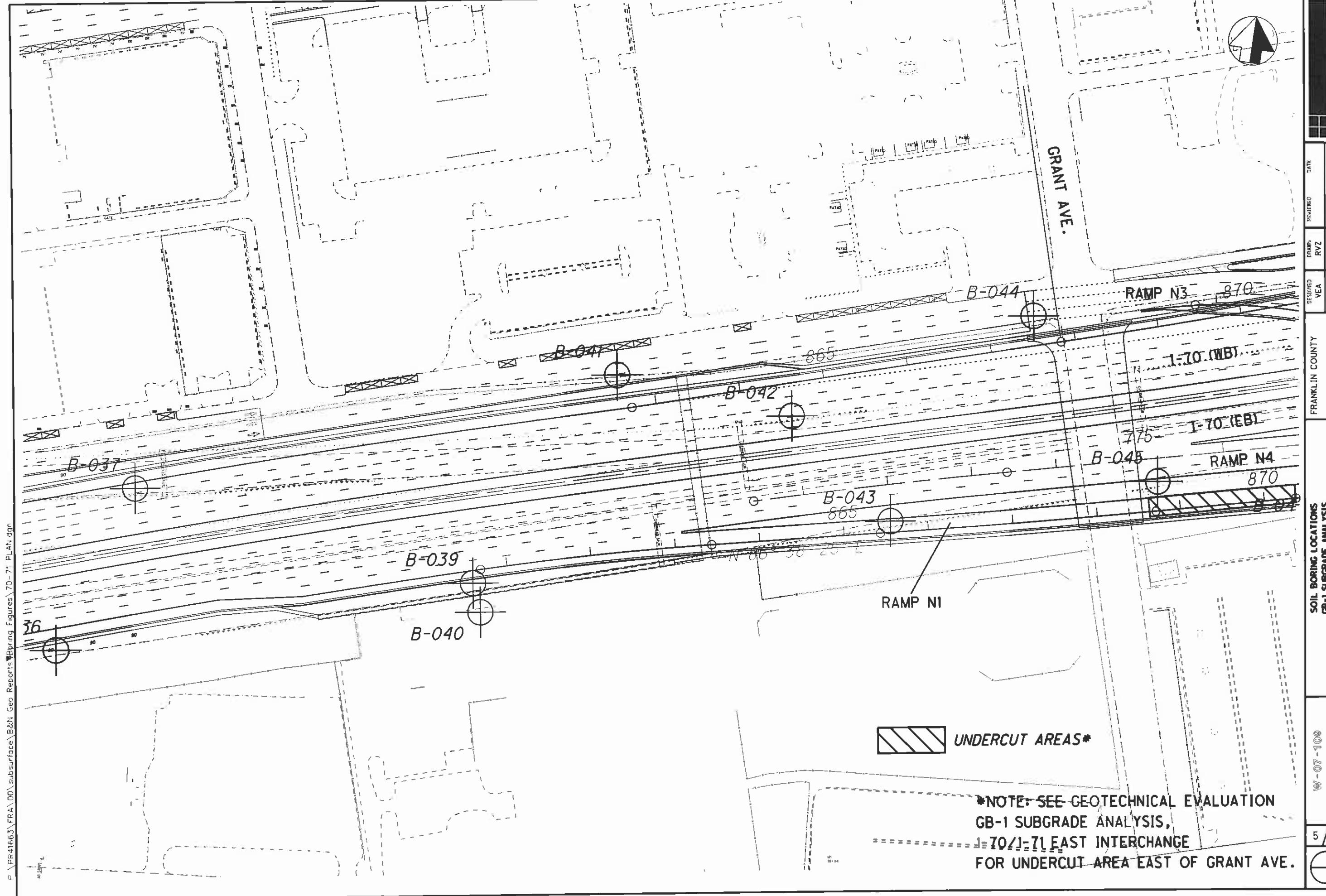












FRANKLIN COUNTY

SOIL BORING LOCATIONS
GB-1 SUBGRADE ANALYSIS
EAST PORTION

W - 07-100
FIGURE 6

5 / 5



DESIGNED BY
DRAWN BY
CHECKED BY
REVISED BY
STRUCTURE FILE NUMBER
Sohn Reed Road
Columbus, Ohio 43220
BUREAU OF HIGHWAYS & SURVEYS

APPENDIX B

TABLES

Table 1
Soil Boring Locations

Project Segment	Station Range	Total Number of Borings	Boring I.D.s
Entire South Trench area	N/A	22	
I-70 (Eastbound)	716+00 – 768+00	11	B-018, B-023, B-024, B-027, B-029, B-031, B-035, B-036, B-039, B-043, B-045
I-70 (Westbound)	716+00 – 768+00	11	B-020, B-022, B-025, B-028, B-030, B-033, B-034, B-037, B-041, B-042, B-044
Ramp F1	263+50 – 267+50	1	B-018
Ramp M1	612+00 – 631+00	1	B-022
Ramp N1	763+00 – 768+20	1	B-043

Soils information from three borings (B-018, B-022, and B-043) was used for subgrade analysis on more than one road segment or ramp.

Table 2
Summary of Topsoil/Existing Pavement Thickness and Observed Water Levels at Boring Locations

Boring I.D.	Topsoil Thickness (inches)	Concrete Thickness (inches)	Asphalt Thickness (inches)	Base Material Thickness (inches)	Depth to Water (feet)	Total Depth (feet)
I-70 Mainline (Eastbound)						
B-018	--	10	--	4	15.0	17.5
B-023	--	12	--	6	19.1	35
B-024	--	--	--	--	16.7	111.5
B-027	--	18	--	11	>14	14
B-029	--	16	--	11	21.0	136.5
B-031	8	--	--	--	9.5	60
B-035	--	18	--	3	9.2	15
B-036	--	20	--	4	14	115.6
B-039	--	16	--	5	>17.5	17.5
B-043	--	18	--	5	14.4	115
B-045	--	8	--	5	>15	15
I-70 Mainline (Westbound)						
B-020	--	16	--	4	>20	20
B-022	--	16	--	4	>20	20
B-025	--	7	--	7	39	59.3
B-028	--	12	--	6	>10	10
B-030	--	11	--	6	13.6	111
B-033	--	--	--	2	9.2	15
B-034	3	--	--	--	43.3	135.5
B-037	--	17	--	5	25.6	130
B-041	--	8	--	5	37.1	135
B-042	2	--	--	--	>15	15
B-044	--	6	--	10	20.8	131.5
Ramp F1						
B-018	--	10	--	4	15.0	17.5
Ramp M1						
B-022	--	16	--	4	>20	20
Ramp N1						
B-043	--	18	--	5	37	115

Soils information from three borings (B-018, B-022, and B-043) was used for subgrade analysis on more than one road segment or ramp.

Table 3
GB 1 Subgrade Analysis Results

Project Segment	Number of Soil Borings	Average N ₆₀	Average N _L	Average PI	Average Moisture	Average Opt. Moist.	Average Group Index (GI)	Design CBR
Entire South Trench Area	22	40.9	22.0	9.9	10.0	10.1	3	9
I-70 Mainline Eastbound	11	40.6	22.6	9.7	10.2	10.2	3	9
I-70 Mainline Westbound	11	41.2	21.3	10.1	10.0	10.0	3	9
Ramp F1**	1							
Ramp M1	1	28.2	13.0	4.7	12.2	10.8	1.5	11
Ramp N1	1	72.3	30.0	8.5	10.0	11.5	4	8

** - The GB1 analysis spreadsheets did not calculate an average Group Index or a Design CBR for Ramp F1 since all the soil samples for that ramp are under approximately 10 feet of fill.

APPENDIX C

GB 1 MODEL ANALYSIS OUTPUT - ENTIRE I-70 SOUTH TRENCH PROJECT AREA

Subgrade Analysis
V. 9.09 08/10/07

Design **9**
CBR
Item 320 Option
Global CS Option
Global LS No

Classification Counts by Sample																	
R	1a	1b	3	3a	2-4	2-5	2-6	2-7	4a	4b	5	6a	6b	7-5	7-6	8a	8b
0	9	14	1	2	2	0	0	0	36	1	0	11	7	0	1	0	0
	11%	17%	1%	2%	2%				43%	1%		13%	8%		1%		
0.0%									33.3%						66.7%		

	N ₆₀	N _L	PI	Clay	M	M _{OPT}	GI
Average	40.9	22.0	9.9	20.8	10.0	10.1	3.05
Maximum	103	30	44	18	28	42	16
Minimum	3	3	14	5	1	3	0

22 Total Borings

Class @ Surface	
2-5	0
4b	0
5	0
7-5	0
7-6	0
8a	0
8b	0
R	0

% Borings	
N _{<= 5}	9%
	9%
N _{<= 10}	14%
	0% 9%
N _{>= 20}	59%
	% Borings
M+	27%
	27%
R	0%

0% 5% 5% 27%

Rig	ER
A	61
B	62
C	63
D	75
E	86
F	79
G	
H	

I-70 South Trench Complete Project Site W-07-109				Standard Penetration				Physical Characteristics				Moisture		Classification		Comments		Problem		Treatments				Analysis		
#	B #	Boring Location	Depth To	Cut Fill	n ₂	n ₃	N _m	Rig	N ₆₀	N _L	LL	PL	PI	% Silt	% Clay	P ₂₀₀	M	M _{OPT}	Class	GI	w/ Class	w/ MN	LS	CS	UC Class	UC MN

1	B-018	70 Eastbound Ramp F1 720+55, 63' R	1 11.2 12.7 2 13.2 14.7 3 15.7 17.2 4 18.2 19.7	9.7	17 12 29 B 6 13 19 20 8 10 18 19 8 5 13 13	33 16 17 30 29 59 19 16 6b 10 16 6b 15 10 4a	fill 9.7 feet first sample 1.5 14" conc. and base	MN	1	No undercut upper 18 feet soils will bridge												
2	B-020	70 Westbound 724+03, 77' L	1 10.7 12.2 2 12.7 14.2 3 15.2 16.7 4 17.7 19.2	9.2	10 12 22 A 17 21 38 39 6 6 12 12 2 3 5 5	23 15 8 37 23 60 13 14 6a 14 14 6a 11 10 4a 12 10 4a	fill 9.2 feet first sample 1.5 20" conc. and base	N	5	No undercut upper soil and fill soils will bridge												
3	B-022	Ramp M1 70 Westbound Sta. 728+60	1 0.0 1.2 2 1.2 2.7 3 2.7 4.2 4 4.2 5.7	-1.8	9 7 16 A 16 7 6 13 13 18 27 45 46 21 16 37 38	15 14 1 21 18 3 28 18 10 30 18 48	29 22 52 18 9 27 13 13 4a 6 1a 0	13 14 6a 11 10 2-4 13 13 4a 10 4a 0	3 cut 1.8 feet 0 20" conc. and base 3 Sample 4 dense 0 gravel, no tests													
4	B-023	70 Eastbound 728+49, 36'R	1 15.3 16.8 2 17.8 19.3 3 20.3 21.8 4 22.8 24.3	14.3	2 3 5 C 5 1 2 3 3 1 2 3 3 2 4 6 6	29 18 11 28 18 10 32 24 56	33 18 51 32 24 56 19 13 4a 21 10 4a	19 14 6a 19 14 6a 19 13 4a 21 10 4a	fill 14.3 feet first sample 1.5 18" conc. and base use sample P1, P2	N MN MN MN	5 5 5 3	No undercut under 14.3 feet of new fill										
5	B-024	70 Eastbound Sta. 730+50	6 0.4 1.9 7 2.9 4.4 8 5.4 6.9 9 7.9 9.4	####	21 22 43 A 44 14 15 29 29 7 7 14 14 4 7 11 11	21 13 8 35 22 57	19 16 6b 16 6b 10 9 10 4a 11 10 4a	11 10 4a 9 10 4a 10 4a 0	10 cut 10.6 feet no lab analysis sample #6, 7, 8													
6	B-025	70 Westbound 732+70	6 0.0 1.4 7 1.9 3.4 8 4.4 5.9 9 6.9 8.4	####	16 17 33 A 34 8 12 20 20 7 12 19 19 6 9 15 15	27 16 11 14 7 7 32 21 52	34 20 54 32 21 52 9 10 4a 10 4a 0	11 14 6a 11 10 4a 9 10 4a 10 4a 0	4 cut 11.6 feet 5 first sample at 8.5' 3 no lab tests 7 and 9													
7	B-027	70 Eastbound 736+64, 51' R	6 0.0 0.6 7 1.6 3.1	####	26 29 55 A 56 27 22 49 50 30	NP NP NP 18 15 3	39 39 9 11	4a 4a 1 9 10 5	1 cut 10.9 feet 5 bottom of boring at 14 feet													
8	B-028	70 Westbound 740+11, 55' L	3 1.2 2.7 4 3.7 5.2	-4.8	20 22 42 A 43 49 46 95 97 30	NP NP NP NP NP NP	17 17 8 6 9 3 6 1a 10 4a 0	1b 1b 0 1a 1a 0 10 4a 0	0 cut 4.8 feet bottom of boring at 10.0 feet													
9	B-029	70 Eastbound 740+42, 86'R	8 0.0 1.2 9 1.2 2.2 10 3.2 3.7 11 5.7 7.2	####	48 45 93 A 95 29 50 79 80 50 50 51 51 28 33 61 62	NP NP NP NP NP NP NP NP NP 30	11 11 3 6 11 11 2 6 6 1a 0 10 2-4	1a 1a 0 1a 1a 0 6 1a 0 10 2-4	0 cut 15.3 feet													
10	B-030	70 Westbound 743+00, 64' L	5 0.0 1.2 6 1.2 2.7 7 2.7 3.7 8 5.2 6.7	-8.3	19 21 40 A 41 15 27 42 43 50 50 51 51 33 38 71 72	23 14 9 21 15 6 NP NP NP 30	41 25 66 10 10 10 12 12 3 6 12 12 3 6	10 10 4a 10 10 4a 1a 1a 0 1a 1a 0	6 cut 8.3 feet 5 no lab tests #6													
11	B-031	70 Eastbound 745+02, 73' R	5 0.5 2.0 6A 2.0 3.5 7 4.5 6.0 8 7.0 8.5	-6.5	21 25 46 C 48 15 17 32 34 12 14 26 27 22 25 47 49	NP NP NP 15 5 10 33 16 48 10 10 4a																

I-70 South Trench Complete Project Site W-07-109							Standard Penetration					Physical Characteristics					Moisture		Classification		Comments		Problem		Treatments				Analysis		
#	B #	Boring Location			Depth To	Cut Fill	n ₂	n ₃	N _m	Rig	N ₆₀	N _L	LL	PL	PI	% Silt	% Clay	P 200	M	M _{OPT}	Class	GI	w/ Class	w/ MN	LS	CS	UC Class	UC MN			
		4	2.4	3.9			20	15	35		37		NP	NP	NP	13		13	10	6	1b	0						1	gravel & sand will bridge		
		5	3.9	5.4			13	15	28		29	29	NP	NP	NP	11		11	12	6	1b	0							2	No undercut upper 4.8' of sand & gravel will bridge	
13	B-034	70 Westbound 752+71, 96' L	11	0.0	1.3	####	11	14	25	C	26		NP	NP	NP	5		5	11	6	1a	0	cut 26.2 feet								
			12	2.3	3.3		19	50	69		72		NP	NP	NP	15		15	8	6	1b	0									
			13	4.8	6.3		4	4	8		8		NP	NP	NP	3		3	7	6	1a	0									
			14	7.3	8.8		30	26	56		59	8	NP	NP	NP	16		16	9	6	1b	0									
14	B-035	70 Eastbound 752+67, 73' R	4	0.0	0.5	-7.0	14	18	32	A	33		NP	NP	NP	8		8	5	8	3	0	cut 7 feet								
			5	0.5	2.0		24	27	51		52		NP	NP	NP	9		9	4	6	1b	0									
			6	2.0	3.5		22	25	47		48		NP	NP	NP	10		10	9	6	1b	0									
			7	4.0	5.5		23	27	50		51	30	NP	NP	NP		9	9	5	6	1b	0									
15	B-036	70 Eastbound 755+56, 101' R	4	0.0	0.5	-7.5	24	38	62	A	63		NP	NP	NP	13		13	5	6	1b	0	cut 7.5 feet								
			5	0.5	2.0		23	25	48		49		NP	NP	NP	12		12	5	6	1b	0	no lab sample 1, 2								
			6	2.0	3.5		32	34	66		67		NP	NP	NP	13		13	5	6	1b	0									
			7	3.5	5.0		25	26	51		52	30	NP	NP	NP	14		14	10	8	3a	0									
16	B-037	70 Westbound 756+81, 69' L	9	0.0	0.9	####	21	22	43	C	45		26	14	12	32	16	49	13	14	6a	3	cut 20.6 feet								
			10	0.9	2.4		24	29	53		56		27	15	12	36	18	54	10	14	6a	5									
			11	2.4	3.9		48	50	98		103		20	12	8	32	15	47	10	10	4a	2	no lab sample 12								
			12	4.4	5.9		24	24	48		50	30	NP	NP	NP				10	10	4a	5									
17	B-039	70 Eastbound 760+57, 102' R	4	0.3	1.8	-7.2	16	19	35	A	36		20	13	7	28	15	43	8	10	4a	2	cut 7.2 feet								
			5	1.8	3.3		14	22	36		37		22	13	9	31	15	45	8	10	4a	2									
			6	3.3	4.8		18	21	39		40		20	13	7	29	16	44	8	10	4a	2									
			7	4.8	5.3		12	15	27		27	27	NP	NP	NP				9	8	3a	0									
18	B-041	70 Westbound 762+59, 116' L	12	0.8	2.3	####	23	34	57	C	60		NP	NP	NP				10	11	4a	5	cut 25.2 feet								
			13	3.3	4.8		24	42	66		69		NP	NP	NP				11	11	4a	5									
			14	8.3	9.8		22	30	52		55		22	13	9	36	19	55	10	10	4a										
			15	13.3	14.8		30	42	72		76	30	NP	NP	NP				10	11	4a										
19	B-042	70 Westbound 764+57, 41' L	3	0.6	2.1	-5.4	14	21	35	C	37		22	13	9	36	22	58	9	10	4a	5	cut 5.4 feet								
			4	2.1	3.1		50	50	53				23	14	9	36	21	57	8	10	4a	4									
			5	3.6	5.1		28	27	55		58								8	10	4a	5									
			6	5.1	5.6		50	50	53		30								9	10	4a										
20	B-043	70 Eastbound and Ramp N1 765+57, 96' R	3	0.4	1.9	-5.6	25	30	55	C	58		NP	NP	NP	38	10	48	12	11	4a	3	first sample 1.9'								
			4	1.9	3.4		12	45	57		60								10	4a	5	no lab sample 4									
			5	3.4	4.9		31	46	77		81		22	16	6	36	22	59	9	11	4a	5									
			6	4.9	6.4		40	46	86		90	30	24	13	11	34</															

APPENDIX D

**GB 1 MODEL ANALYSIS OUTPUT - INDIVIDUAL ROADWAY SEGMENTS
FOR THE I-70 SOUTH TRENCH**

Subgrade Analysis
V. 9.09 08/10/07

Design **9**
CBR
Item 320 Option
Global CS Option
Global LS No

Classification Counts by Sample																	
R	1a	1b	3	3a	2-4	2-5	2-6	2-7	4a	4b	5	6a	6b	7-5	7-6	8a	8b
0	3	7	1	2	1	0	0	0	17	1	0	5	5	0	0	0	0
	7%	17%	2%	5%	2%				40%	2%		12%	12%				
0.0%									33.3%						66.7%		

		N ₆₀	N _L	PI	Clay	M	M _{OPT}	GI
		Average						
		40.6	22.6	9.7	19.0	10.2	10.2	2.48
		95	30	37	18	67	21	10
		3	3	15	5	8	2	0

11 Total Borings

Class @ Surface	
2-5	0
4b	0
5	0
7-5	0
7-6	0
8a	0
8b	0
R	0

% Borings	
N _f <= 5	9%
18%	
N _f <= 10	9%
0% 18%	
N _f >= 20	64%
% Borings	
M+	27%
27%	
R	0%

% Surface	
0%	0%
9%	27%

Rig	ER
A	61
B	62
C	63
D	75
E	86
F	79

I-70 South Trench (70 Eastbound) W-07-109				Standard Penetration				Physical Characteristics				Moisture	Classification	Comments	Problem	Treatments		Analysis							
#	B #	Boring Location	Depth To	Cut Fill	n ₂	n ₃	N _m	Rig	N ₆₀	N _L	LL	PL	PI	% Silt	% Clay	P 200	M	M _{OPT}	Class	GI	w/ Class	w/ MN	LS	CS Class	UC MN

1	B-018	Ramp F1 70 Eastbound 720+55, 63' R	1 2 3 4	11.2 12.7 13.2 14.7 15.7 17.2 18.2 19.7	9.7	17 6 8 8	12 13 18 13	29 20 19 13	B C C C	30 19 16 13	33 16 17	30 29 59	10 19 16 15	16 6b 6b 4a	fill 9.7 feet first sample 1.5 14" concrete and base						No undercut upper 18 feet of soil will bridge				
2	B-023	70 Eastbound 728+49, 36'R	1 2 3 4	15.3 17.8 17.8 19.3 20.3 21.8 22.8 24.3	14.3	2 1 1 2	3 2 3 6	5 3 3 3	C C C C	5 5 5 3	29 28 18 10	33 32 18 10	18 24 56	14 19 14 21	6a 6a 6a 10	fill 14.3 feet first sample 1.5 feet 18" conc and base						No undercut under 14.3 feet of new fill			
3	B-024	70 Eastbound Sta. 730+50	6 7 8 9	0.4 1.9 2.9 4.4 5.4 6.9 7.9 9.4	####	21 14 7 4	22 15 7 7	43 29 14 11	A A A A	44 29 29 11	21 13 8	35 22 57	19 16 16 11	6b 6b 4a 4a	10 10 10 10	cut 10.6 feet no lab analysis sample #6, 7, 8									
4	B-027	70 Eastbound 736+64, 51'R	6 7	0.0 0.6 1.6 3.1	####	26 27	29 22	55 49	A A	56 50	NP 18	NP 15	NP 3	9 9	11 10	4a 4a	1 5	cut 10.9 feet bottom of boring at 14 feet							
5	B-029	70 Eastbound 740+42, 86' R	8 9 10 11	0.0 1.2 1.2 2.2 3.2 3.7 5.7 7.2	####	48 29 50 28	45 50 50 33	93 79 50 61	A A A A	95 80 51 62	NP NP NP NP	NP NP NP NP	NP NP NP NP	11 11 11 30	11 11 11 11	3 2 6 10	1a 1a 1a 2-4	0 0 0 0	cut 15.3 feet						
6	B-031	70 Eastbound 745+02, 73' R	5 6A 7 8	0.5 2.0 2.0 3.5 4.5 6.0 7.0 8.5	-6.5	21 15 12 22	25 17 14 25	46 32 26 47	C C C C	48 34 27 49	NP 15 5 24	NP 5 10	NP 10	33 16 48	18 8 10 10	6 4a 4b 4a	0 3 3 0	cut 6.5 feet						No undercut 4.5' of A-1b soil w/46 b/ft. A-4b > 3', o.k.	
7	B-035	70 Eastbound 752+67, 73'R	4 5 6 7	0.0 0.5 0.5 2.0 2.0 3.5 4.0 5.5	-7.0	14 24 22 23	18 27 25 27	32 51 47 50	A A A A	33 52 48 51	NP NP NP NP	NP NP NP NP	NP NP NP NP	8 9 10 9	5 4 6 5	8 6 6 6	3 1b 1b 1b	0 0 0 0	cut 7 feet						
8	B-036	70 Eastbound 755+56, 101'R	4 5 6 7	0.0 0.5 0.5 2.0 2.0 3.5 3.5 5.0	-7.5	24 23 32 25	38 25 34 26	62 48 66 51	A A A A	63 49 67 52	NP NP NP NP	NP NP NP NP	NP NP NP NP	13 12 13 14	5 5 5 10	6 6 6 8	1b 1b 1b 3a	0 0 0 0	cut 7.5 feet no lab sample 1,2						
9	B-039	70 Eastbound 760+57, 102' R	4 5 6 7	0.3 1.8 1.8 3.3 3.3 4.8 4.8 5.3	-7.2	16 14 18 12	19 22 21 15	35 36 39 27	A A A A	36 37 40 27	NP NP NP NP	NP NP NP NP	NP NP NP NP	28 31 37 29	15 13 16 16	43 46 45 45	8 8 8 9	10 10 10 8	4a 4a 4a 3a	2 2 2 0	cut 7.2 feet				

Subgrade Analysis

V. 9.09 08/10/07

Design CBR 9

Item 320 Option
Global CS Option
Global LS No

11 Total Borings

Classification Counts by Sample																	
R	1a	1b	3	3a	2-4	2-5	2-6	2-7	4a	4b	5	6a	6b	7-5	7-6	8a	8b
0	6	7	0	0	1	0	0	0	19	0	0	6	2	0	1	0	0
	14%	17%			2%				45%		14%	5%		2%			
	0.0%				33.3%							66.7%					

Class @ Surface	
2-5	0
4b	0
5	0
7-5	0
7-6	0
8a	0
8b	0
R	0

% Borings	
N _L <= 5	9%
	0%
N _L <= 10	18%
	0% 0%
N _L >= 20	55%
	% Borings
M+	27%
R	0%

% Surface	
N _L <= 5	9%
	0%
N _L <= 10	18%
	0% 0%
N _L >= 20	55%
	% Borings
M+	27%
R	0%

Rig	ER
A	61
B	62
C	63
D	75
E	86
F	79
G	
H	

I-70 South Trench 70 Westbound W-07-109				Standard Penetration				Physical Characteristics				Moisture		Classification		Comments		Problem		Treatments		Analysis				
#	B #	Boring Location	Depth To	Cut Fill	n ₂	n ₃	N _m	Rig	N ₆₀	N _L	LL	PL	PI	% Silt	% Clay	P 200	M	M _{OPT}	Class	GI	w/ Class	w/ MN	LS	CS Class	UC MN	

1	B-020	70 Westbound 724+03, 77'L	1	10.7	12.2	9.2	10	12	22	A	22						13	14	6a		fill 9.2 feet first sample 1.5	N				5	No undercut upper soil and fill soils will bridge
2	B-022	70 Westbound Ramp M1 Sta. 728+60	1	0.0	1.2	-1.8	9	7	16	A	16	15	14	8	37	23	60	11	10	6a	3 cut 1.8 feet						
2	B-022	70 Westbound Ramp M1 Sta. 728+60	2	1.2	2.7		7	6	13		13	21	18	3	18	9	27	11	10	2-4	0 20" conc. And base						
2	B-022	70 Westbound Ramp M1 Sta. 728+60	3	2.7	4.2		18	27	45		46	28	18	10	30	18	48	13	13	4a	3 Sample 4 dense						
2	B-022	70 Westbound Ramp M1 Sta. 728+60	4	4.2	5.7		21	16	37		38	13						6	1a	0 gravel, no tests							
3	B-025	70 Westbound 732+70	6	0.0	1.4	####	16	17	33	A	34	27	16	11	34	20	54	11	14	6a	4 cut 11.6 feet						
3	B-025	70 Westbound 732+70	7	1.9	3.4		8	12	20		20							11	10	4a	5 first sample at 8.5'						
3	B-025	70 Westbound 732+70	8	4.4	5.9		7	12	19		19	14	32	21	53	9	10	10	4a	4 no lab tests 7 nd 9							
4	B-028	70 Westbound 740+11, 55'L	3	1.2	2.7	-4.8	20	22	42	A	43	NP	NP	NP	17		8	6	1b	0 cut 4.8 feet							
4	B-028	70 Westbound 740+11, 55'L	4	3.7	5.2		49	46	95		97	NP	NP	NP	9		9	3	1a	0 bottom of boring at 10.0 feet							
5	B-030	70 Westbound 743+00, 64' L	5	0.0	1.2	-8.3	19	21	40	A	41	23	14	9	41	25	66	10	10	4a	6 cut 8.3 feet						
5	B-030	70 Westbound 743+00, 64' L	6	1.2	2.7		15	27	42		43	21	15	6	12		12	1	6	1a	5 no lab tests #6						
5	B-030	70 Westbound 743+00, 64' L	7	2.7	3.7		50	50	51		51	NP	NP	NP	12		12	3	6	1a	0						
6	B-033	70 Westbound 749+82, 43'L	2	0.0	0.9	-4.1	16	19	35	C	37	NP	NP	NP	11		11	4	6	1b	0 cut 4.1 feet	M				1	No undercut top 4' of dense gravel & sand will bridge
6	B-033	70 Westbound 749+82, 43'L	3	0.9	2.4		21	28	49		51	NP	NP	NP	13		13	10	6	1b	0 no lab sample #2						
6	B-033	70 Westbound 749+82, 43'L	4	2.4	3.9		20	15	35		37	NP	NP	NP	11		11	12	6	1b	0						
6	B-033	70 Westbound 749+82, 43'L	5	3.9	5.4		13	15	28		29	NP	NP	NP	11		11	12	6	1b	0						
7	B-034	70 Westbound 752+71, 96'L	11	0.0	1.3	####	11	14	25	C	26	NP	NP	NP	5		5	11	6	1a	0 cut 26.2 feet	N				2	No undercut upper 4.8' of sand & gravel will bridge
7	B-034	70 Westbound 752+71, 96'L	12	2.3	3.3		19	50	69		72	NP	NP														

Subgrade Analysis

v. 9.09 08/10/07

08/10/07

Class @ Surface	
2-5	0
4b	0
5	0
7-5	0
7-6	0
8a	0
8b	0
R	0

% Borings	
$N_L \leq 5$	0%
$N_L \leq 10$	0%
$N_L \geq 20$	0%
M+	100%
R	0%

% Surface	
0%	0%
0%	0%
% Borings	
100%	100%
0%	0%

ig	ER
A	61
B	62
C	63
D	75
E	86
F	79
G	
H	

Design
CBR
Item 320 No
Global CS Option
Global LS No

1 | Total Borings

	N_{60}	N_L	PI		Clay		M	M_{OPT}	GI
Average	20.5	13.0		17.0		29.0		13.5	14.5
Maximum	30	13	33	16	17	30	29	59	19
Minimum	13	13	33	16	17	30	29	59	10

Subgrade Analysis

V. 9.09 08/10/07

Design CBR 11
Item 320 No
Global CS Option
Global LS No

Classification Counts by Sample																	
R	1a	1b	3	3a	2-4	2-5	2-6	2-7	4a	4b	5	6a	6b	7-5	7-6	8a	8b
0	1	0	0	0	1	0	0	0	1	0	0	1	0	0	0	0	0
	25%				25%				25%			25%					
	0.0%				50.0%							50.0%					

N ₆₀	N _L	PI	Clay	M	M _{OPT}	GI
Average		4.7	16.5	12.2	10.8	1.50
Maximum		28	18	51.6	12.9	14
Minimum		15	14	9	26.9	11

1 Total Borings

I-70 South Trench Ramp M1 W-07-109															Standard Penetration		Physical Characteristics					Moisture		Classification		Comments		Problem		Treatments				Analysis	
#	B #	Boring Location		Depth To	Cut Fill	n ₂	n ₃	N _m	Rig	N ₆₀	N _L	LL	PL	PI	% Silt	% Clay	P ₂₀₀	M	M _{OPT}	Class	GI	w/ Class	w/ MN	LS	CS	UC Class	UC MN								
1	B-022	Ramp M1 70 Westbound Sta. 728+60	1	0.0 1.2	-1.8	9	7	16	A	16		15	14	1	29	22	52	13	14	6a	3	cut 1.8 feet 0 20" conc. and base													
			2	1.2 2.7		7	6	13		13		21	18	3	18	9	27	11	10	2-4	0														
			3	2.7 4.2		18	27	45		46		28	18	10	30	18	48	13	13	4a	3	Sample 4 dense													
			4	4.2 5.7		21	16	37		38	13							6	1a	0	gravel, no tests														
2																																			
3																																			
4																																			
5																																			
6																																			
7																																			
8																																			
9																																			
10																																			
11																																			
12																																			

Class @ Surface	
2-5	0
4b	0
5	0
7-5	0
7-6	0
8a	0
8b	0
R	0

% Borings	
N _L <= 5	0%
N _L <= 10	0%
N _L >= 20	0%
M+	0%
R	0%

% Surface	
0%	0%
0%	0%
0%	0%
0%	0%

Rig	ER
A	61
B	62
C	63
D	75
E	86
F	79
G	
H	

Subgrade Analysis

V. 9.09 08/10/07

08/10/07

Design CBR	8
Item 320	Option
Global CS	Option
Global LS	No

Class @ Surface
2-5 0
4b 0
5 0
7-5 0
7-6 0
8a 0
8b 0
R 0

% Borings	
$N_L \leq 5$	0%
$N_L \leq 10$	0%
$N_L >= 20$	100%
M+	0%
R	0%

% Surface	
0%	
0%	0%
% Borings	
0%	
0%	0%

	ER
	61
	62
	63
	75
	86
	79

APPENDIX E

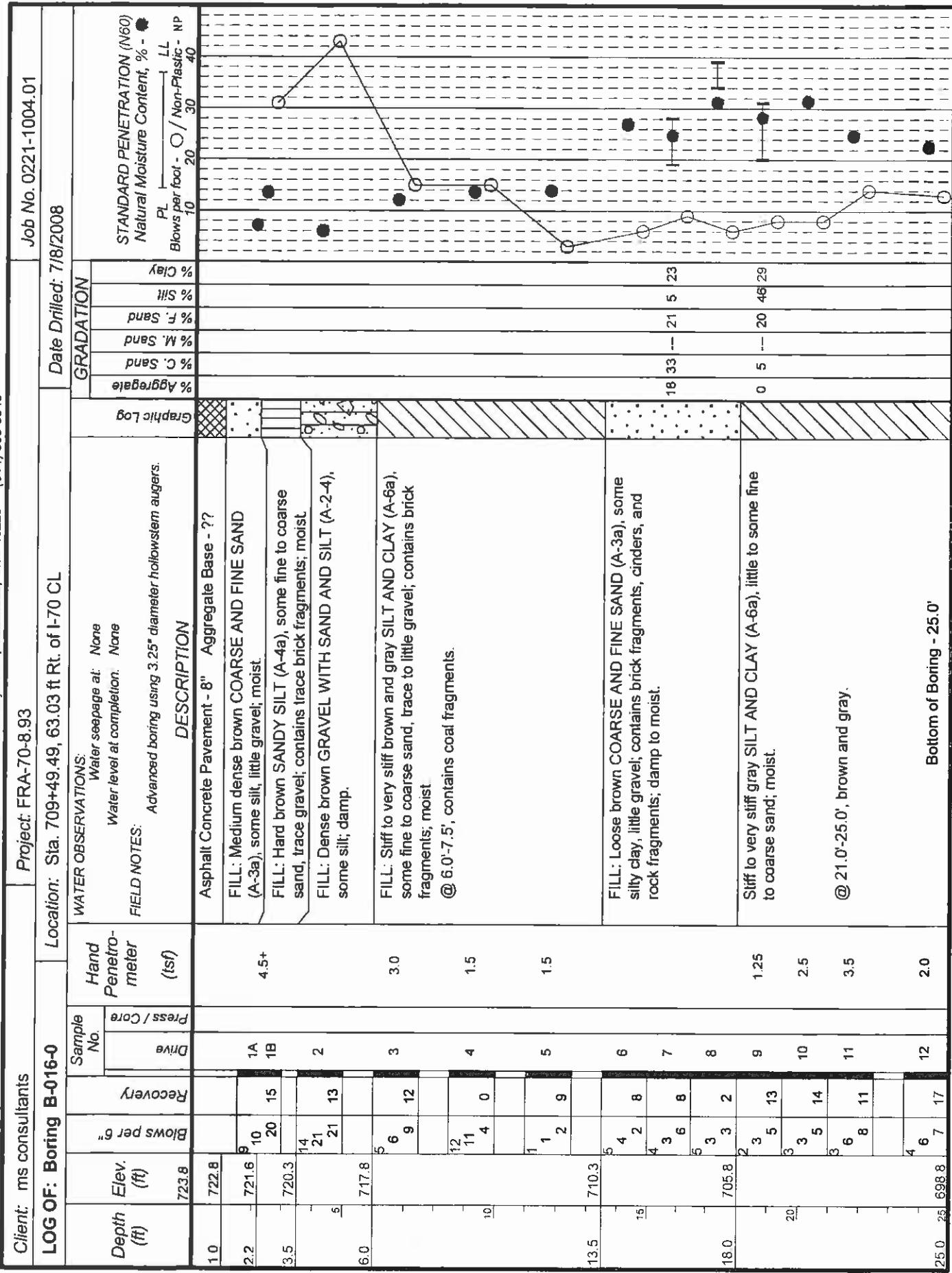
**BORING LOGS
I-70 SOUTH TRENCH**

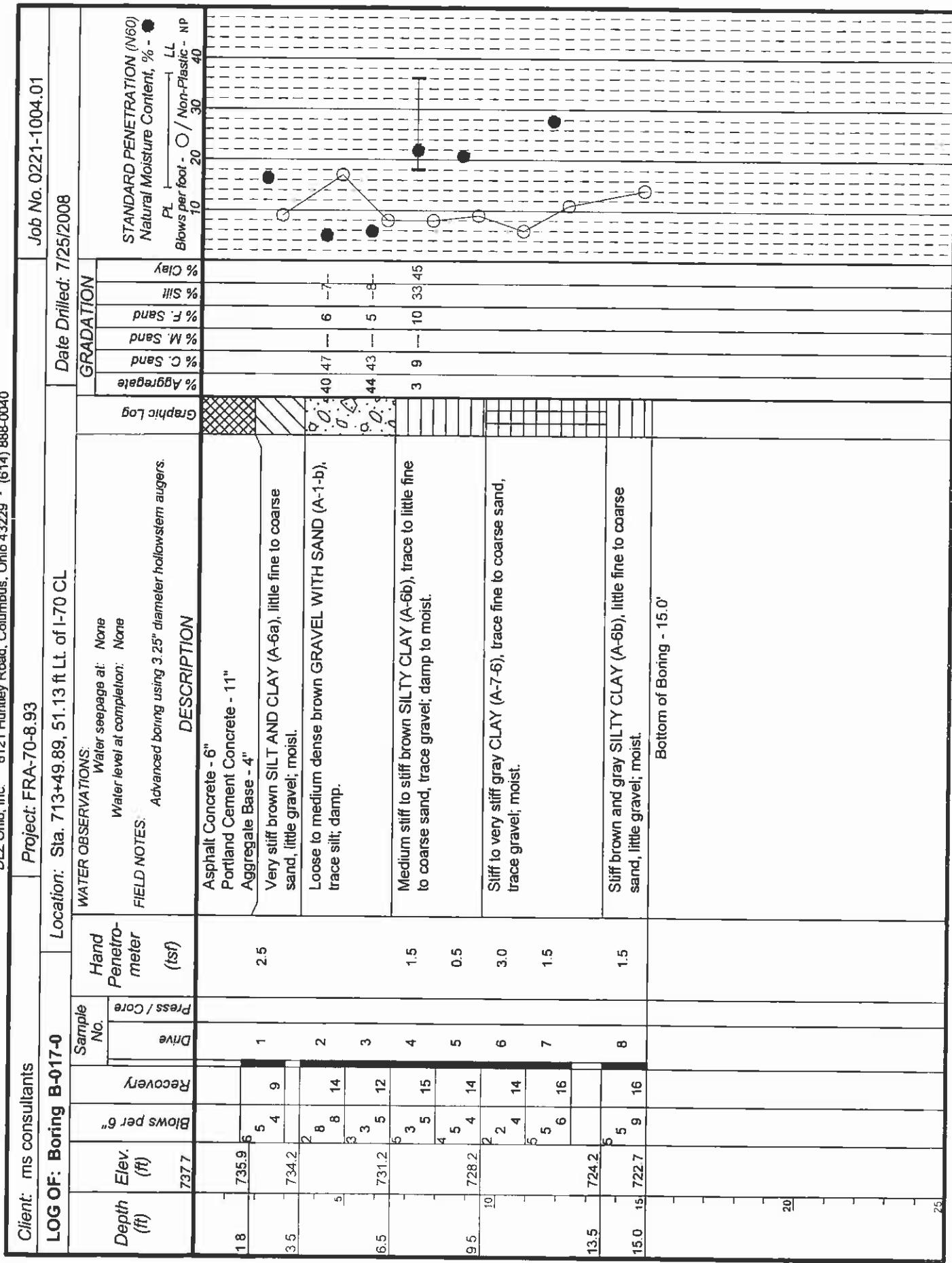
Project: FRA-70-8.93

LOG OF: Boring B-016-0 Location: Sta. 709+49 49 63 0

UNITED CONFEDERATIONS

GRADUATION

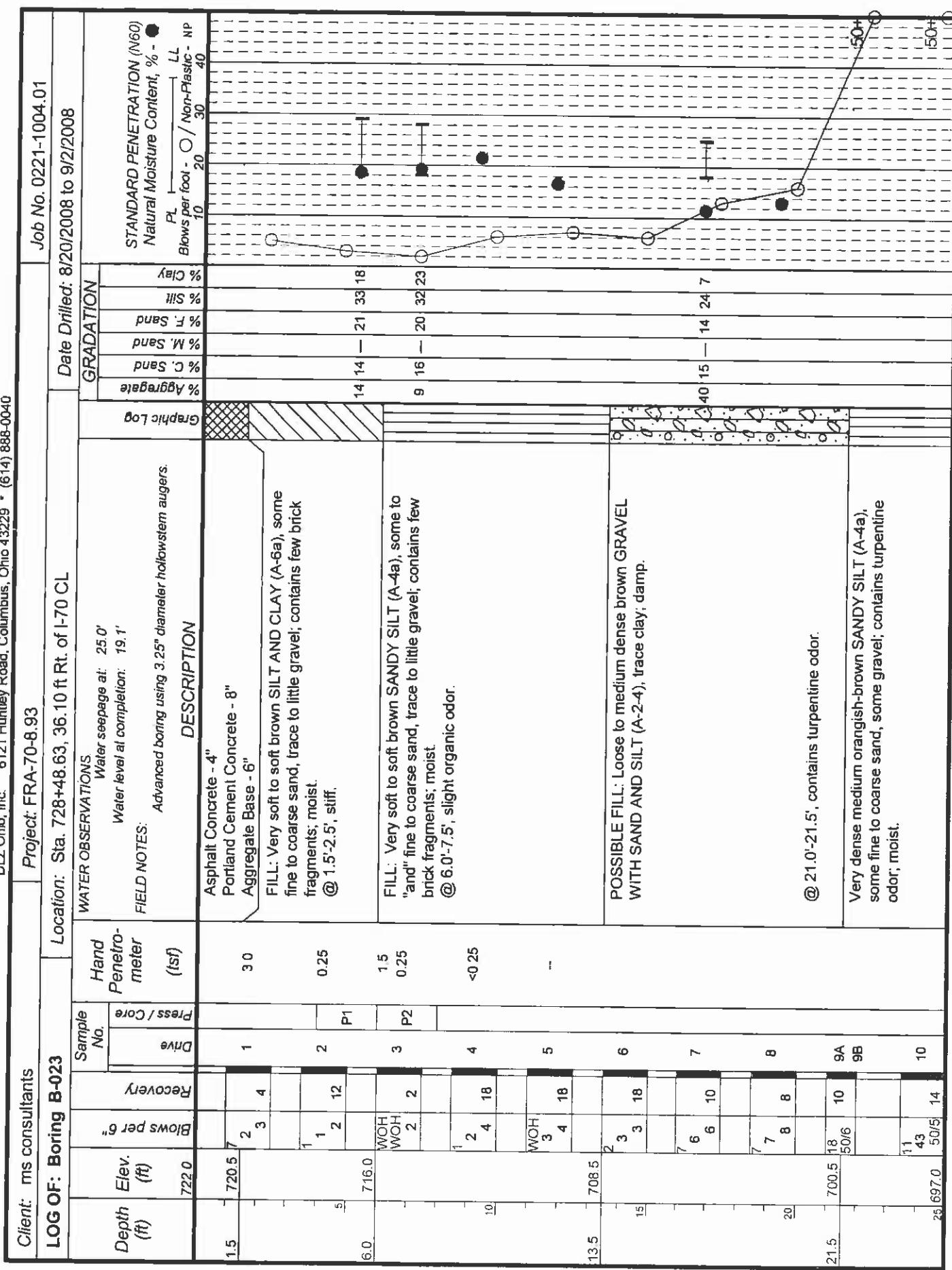




Client: rms consultants		Project: FRA-70-8.93		Job No. 0221-1004.01	
LOG OF: Boring B-018		Location: Sta. 720+55.54, 63.68 ft Rt. of I-70 CL		Date Drilled: 7/8/2008	
Depth (ft)		Sample No.		WATER OBSERVATIONS	
1.2	739.2	Hand Penetrometer (tsf)	Water seepage at: None Water level at completion: 15.0'	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL - Blows per foot - ○ / Non-Plastic - NP LL - 40 CL - 30
5	740.4	Blows per 6"	Drive	DESCRIPTION	GRADATION
7.0	733.4	Recovery	Press / Core	FILL: Very stiff brown SILTY CLAY (A-6b), little to some fine to coarse sand, little gravel; contains rock fragments; moist.	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
8.5	731.9	Blows per 6"	Drive	FILL: Medium dense brown SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
10	729.4	Recovery	Press / Core	FILL: Stiff gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; contains brick fragments; moist.	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
11.0	722.9	Blows per 6"	Drive	Stiff to very stiff gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; moist.	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
15		Recovery	Press / Core	@ 16.0-17.5', contains wood fragments.	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
17.5		Blows per 6"	Drive	Bottom of Boring - 17.5'	% Clay % Silt % Sand % M. Sand % C. Sand % F. Sand % Aggregates
					20

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 724+03.63, 77.13 ft Lt. of I-70 CL		Date Drilled: 7/25/2008		Job No. 0221-1004.01	
LOG OF: Boring B-020									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	Water seepage at:	None	GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL - Blows per foot - ○ / Non-Plastic - NP LL - 30 CLL - 40		
1.7	733.1	5	3.0	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.					
		10	1	Asphalt Concrete - 6"					
		12		Portland Cement Concrete - 10"					
		6	2	Aggregate Base - 4"					
		17		Very stiff brown SILT AND CLAY (A-6a), some fine to coarse sand, little gravel; moist.					
5		21	9						
6.0	728.8	4	3	Very stiff gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.					
		6	2.0						
		6	18						
		10	4	Very stiff gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.					
		2	2.25	@ 8.5'-10.0', contains trace brick fragments; probably carried by spoon from higher elevation.					
		3	4						
		13							
		5	5						
		14	2.0						
		14	9						
		15	6						
		18	1						
15		18	1						
16.0	718.8	10	7	Very stiff grayish brown SILT AND CLAY (A-6a), little fine to coarse sand, little gravel; very slightly organic, trace organic odor, damp.					
		15	7						
		17	11						
		14	8						
20.0	714.8	14	18	Bottom of Boring - 20.0'					
		15							
		14							

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 728+59.94, 91.57 ft Lt. of I-70 CL		Date Drilled: 7/24/2008	
LOG OF: Boring B-022							
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetrometer (tsf)	Press / Core Drive	GRADATION
-	736.3	6	9	1	1.5		STANDARD PENETRATION (N60) Natural Moisture Content, % - PL - Blows per foot - O / Non-Plastic - NP 10 20 30 40 LL - 30
3.0	735.0	7	12	3	-		% Clay
4.5	733.5	6	7	2	-		% Silt
6.0	732.0	27	15	3	2.0		% F. Sand
7.5	730.5	21	16	4	9		% M. Sand
10.0	727.0	8	14	6	3.0		% C. Sand
11.0		27	11	7			% Aggregate
15.0		6	3	1			Graphic Log
16.0	722.0	5	8	9			
18.5	719.5	4	5	13			
20.0	718.0	5	6	14	2.25		
							Date Drilled: 7/24/2008
							Job No. 0221-1004.01



Client: ms consultants		Project: FRA-70-8.93		Job No. 0221-1004.01	
LOG OF: Boring B-023		Location: Sta. 728+48.63, 36.10 ft Rt. of I-70 CL		Date Drilled: 8/20/2008 to 9/2/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS	
				Water seepage at: 25.0' Water level at completion: 19.1'	
69.70	68.00	18	11	Advanced boring using 3.25" diameter hollowstem augers.	
		32	18	DESCRIPTION	
		33	18	Very dense brown COARSE AND FINE SAND (A-3a), some silt, some gravel, wet.	
		7	12	@ 26.0'-27.5', contains turpentine odor.	
		31	15		
		36	15		
		29	17		
		18	13		
		23	17		
		29	17		
		18	14		
		28	15		
		35	15		
35.0	35	687.0	15	Bottom of Boring - 35.0'	

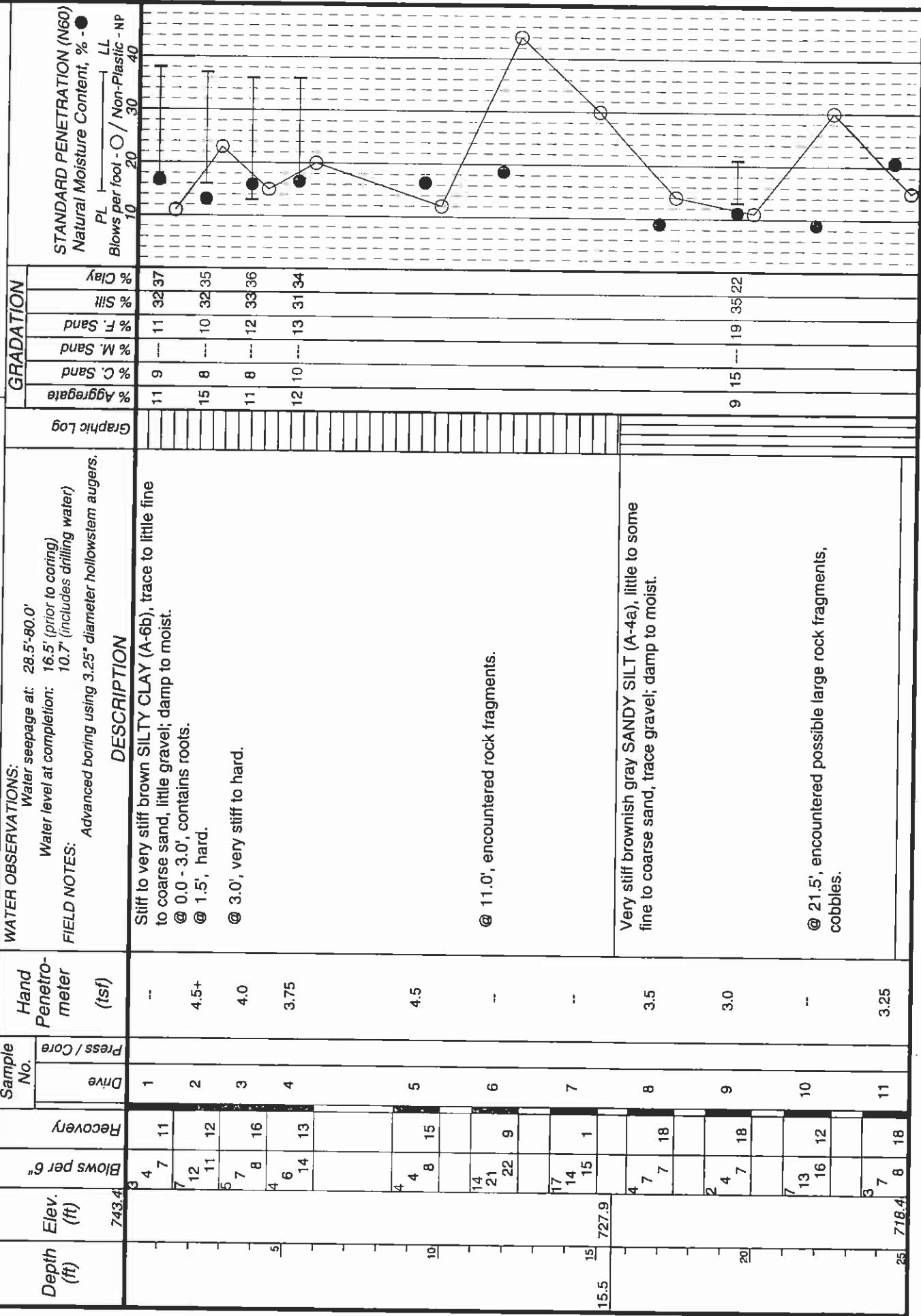
Client: ms consultants

LOG OF: Boring B-024

Project: FRA-70-8.93

Location: Sta. 730+58.01, 93.53 ft Rt. of I-70 CL

Date Drilled: 7/1/2008 to 7/2/2008



Project: FRA-70-8.93

Job No. 0221-1004.01

Client: ms consultants

LOG OF: Boring B-024

Location: Sta. 730+58.01, 93.53 ft Rt. of I-70 CL

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetro- meter (fsf)	WATER OBSERVATIONS:		FIELD NOTES: <i>Advanced boring using 3.25" diameter hollowstem augers.</i>	DESCRIPTION	GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % ●	Blows per foot - O / Non-Plastic - NP PL LL		
						Press / Core	Drive			% Clay	% Silt	% M. Sand	% C. Sand	% F. Sand	% Aggregates
28.0	715.4	718.4	8	10	--	12	14	18	Very stiff brownish gray SANDY SILT (A-4a), little to some fine to coarse sand, trace gravel; damp to moist.	NP	++	0	0	---	-83--
30			3	12	--	13	15	16	Medium dense brownish gray SILT (A-4b), little fine sand; wet.	NP	++	0	0	---	-83--
32.0	711.4		7	18	--	14	18	18	Dense brown GRAVEL (A-1-a), some fine to coarse sand, trace to little silty clay; wet.	60	++	22	8	---	-10--
35										0	0	0	0	0	0
37.0	706.4								Hard brownish gray SANDY SILT (A-4a), little gravel, trace clay; damp.	12	14	27	33	14	--
40										0	0	0	0	0	0
42.0	701.4									0	0	0	0	0	0
45										0	0	0	0	0	0
47.0	696.4								Very dense brownish gray FINE SAND (A-3), trace silty clay; wet. @ 43.5', 2.0 feet sand heave.	16	25	47	47	47	--
50	693.4								Very dense brownish gray SANDY SILT (A-4a), little gravel; wet.	26	30	27	13	17	--

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-024		Location: Sta. 730+58.01, 93.53 ft Rt. of I-70 CL		Date Drilled: 7/1/2008 to 7/2/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (sf)	WATER OBSERVATIONS:	
		Press / Core	Drive	Water seepage at: 28.5'-80.0' Water level at completion: 16.5' (prior to coring) FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	
55	693.4	Recovery	Blows per 6"	@ 50.0-60.0', difficulty advancing boring due to obstruction inside augers blocking rods; possible boulder zone.	
57.0	686.4	15 40 50/4	18 13	Very dense brownish gray SANDY SILT (A-4a), little gravel; wet.	
60	681.4	37 21 50	19 13	Very dense brownish gray SANDY SILT (A-4a), some gravel, some fine to coarse sand; damp.	
62.0	681.4	11 50/4	20	Very dense brown and gray GRAVEL (A-1-a), some fine to coarse sand, trace silt; wet. @ 63.5', one foot sand heave; encountered black shale fragments.	
65		50/3	0	@ 68.5', possible cobbles or boulders.	
70		50/3	6	@ 73.5', 6.0 feet sand heave; washed out with tricone.	
75	668.4	50/3	22	50+	

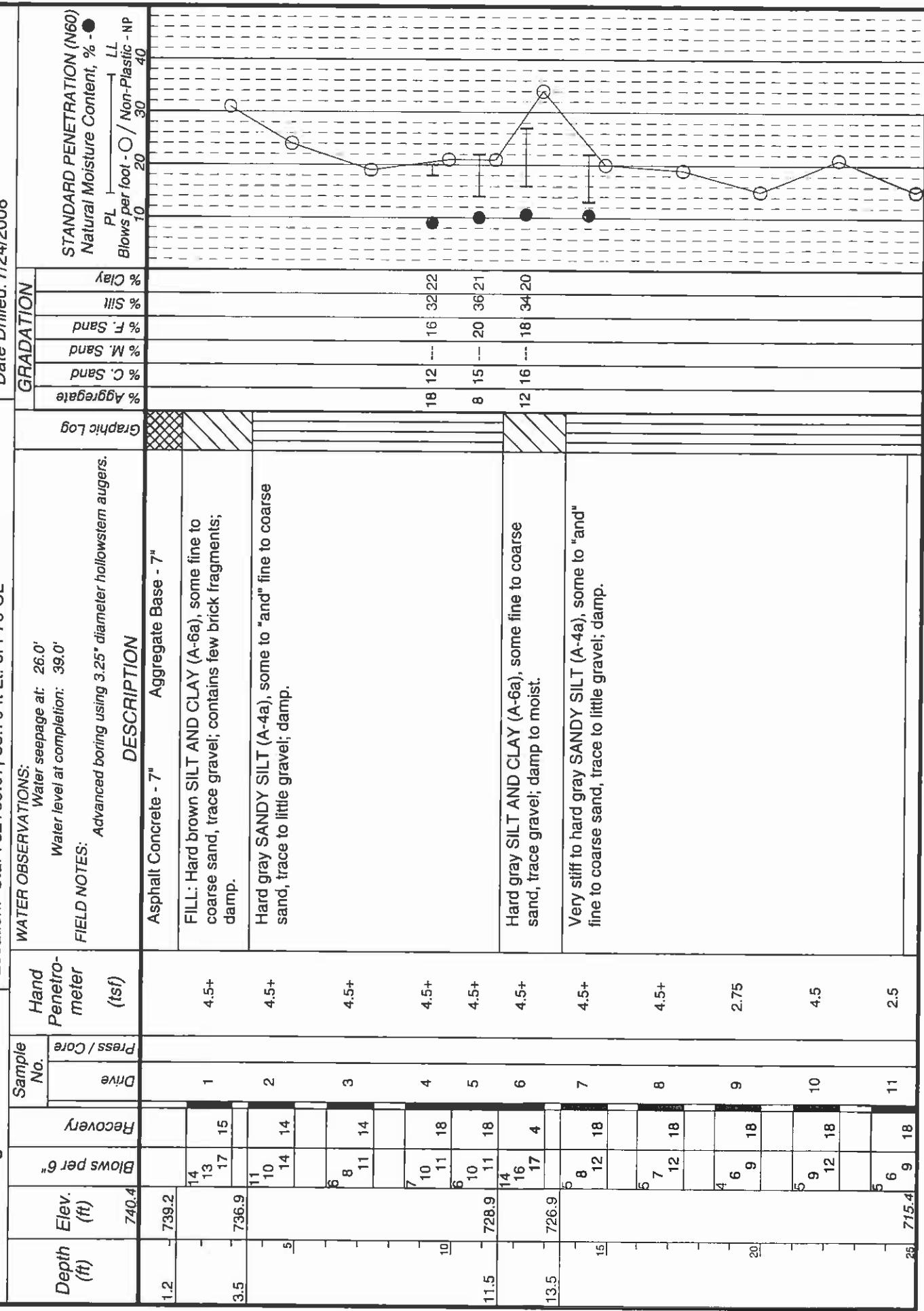
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-025

Location: Sta. 732+65.07, 88.70 ft Lt. of I-70 CL

Job No. 0221-1004.01



Client: ms consultants

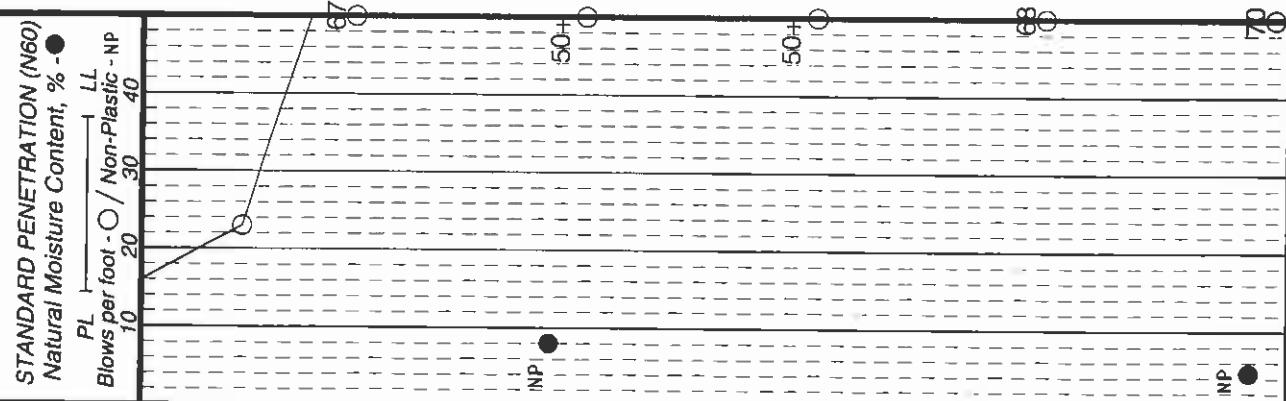
Project: FRA-70-8.93

LOG OF: Boring B-025

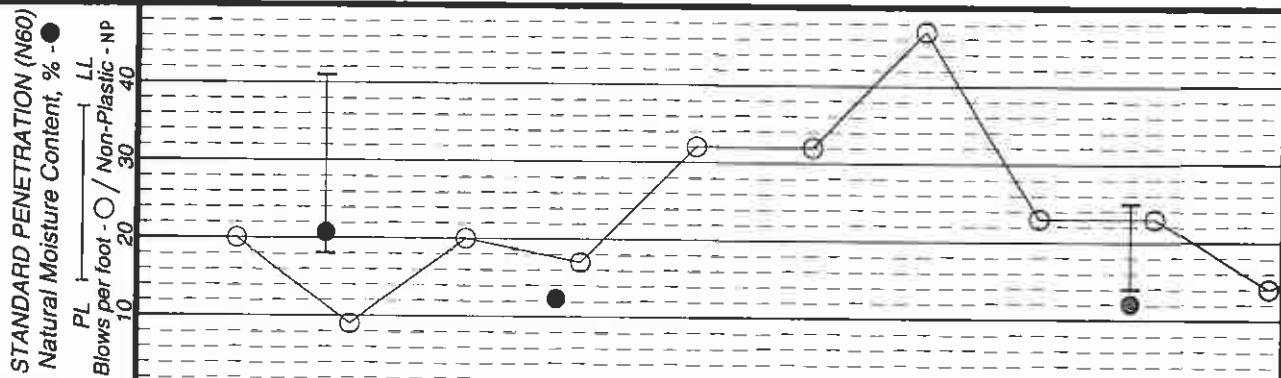
Location: Sta. 732+65.07, 88.70 ft Lt. of I-70 CL

Job No. 02221-1004.01

Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem auger.	DESCRIPTION	GRADATION						Date Drilled: 7/24/2008	
				Blows per 6"	Recovery			Drive	Press / Core	% C.I.	% S.I.	% M. Sand	% C. Sand	% Non-Plastic - NP	
28.5	711.9	3	6	17	13	12	3.5								
		6	17	13	13										
30		29	29	10	13										
		37	37	10	13										
35															
38.5	701.9	29	50/5	6	14										
		50/6	6	14											
40															
43.5	696.9	23	50/6	10	15		4.5+								
		30	30	12	16										
45		37	37	12	16										
50	690.4	22	39	30	15										
		39	39	30	15										



Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL		Date Drilled: 8/1/2008 to 8/6/2008	Job No. 0221-1004.01
LOG OF: Boring B-026						GRADATION	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (fsf)	WATER OBSERVATIONS:		STANDARD PENETRATION (N60)	
		Drive	Press / Core	Water seepage at: 41.0', 58.0', 77.0' Water level at completion: 29.1' (beginning of shift, 8/5/08) 30.7' (includes drilling water)		PL Blows per foot - ○ / Non-Plastic - NP LL	Natural Moisture Content, % - ●
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.	Description	% Clay	% Silt
0.3	753.7	8	11	Topsoil - 4"	Stiff brown SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; moist.	5	6 --- 37 39
2.0	752.0	8	18	1A	1.5	5	6 --- 37 39
3.5	750.5	5	4	1B	0.5	5	6 --- 37 39
5		5	18		2.75	5	6 --- 37 39
		5	8		2	5	6 --- 37 39
		5	11		4.25	5	6 --- 37 39
		11	8		0.5	5	6 --- 37 39
		10	8		@ 8.5'-10.0', soft, wet.	5	6 --- 37 39
		8	18			5	6 --- 37 39
		8	10			5	6 --- 37 39
		10	20			5	6 --- 37 39
		5	10			1.5	6 --- 37 39
		10	20			1.5	6 --- 37 39
		19	23			---	6 --- 37 39
		23	22			---	6 --- 37 39
		18.5	735.5			---	6 --- 37 39
		11	8			3.0	6 --- 37 39
		8	14			2.5	6 --- 37 39
		11	12			10	4.5+
		12	10			10	4.5+
		7	5			8	4.5+
		25	729.0			8	4.5+



Very stiff to hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; damp to moist.

Client: ms consultants

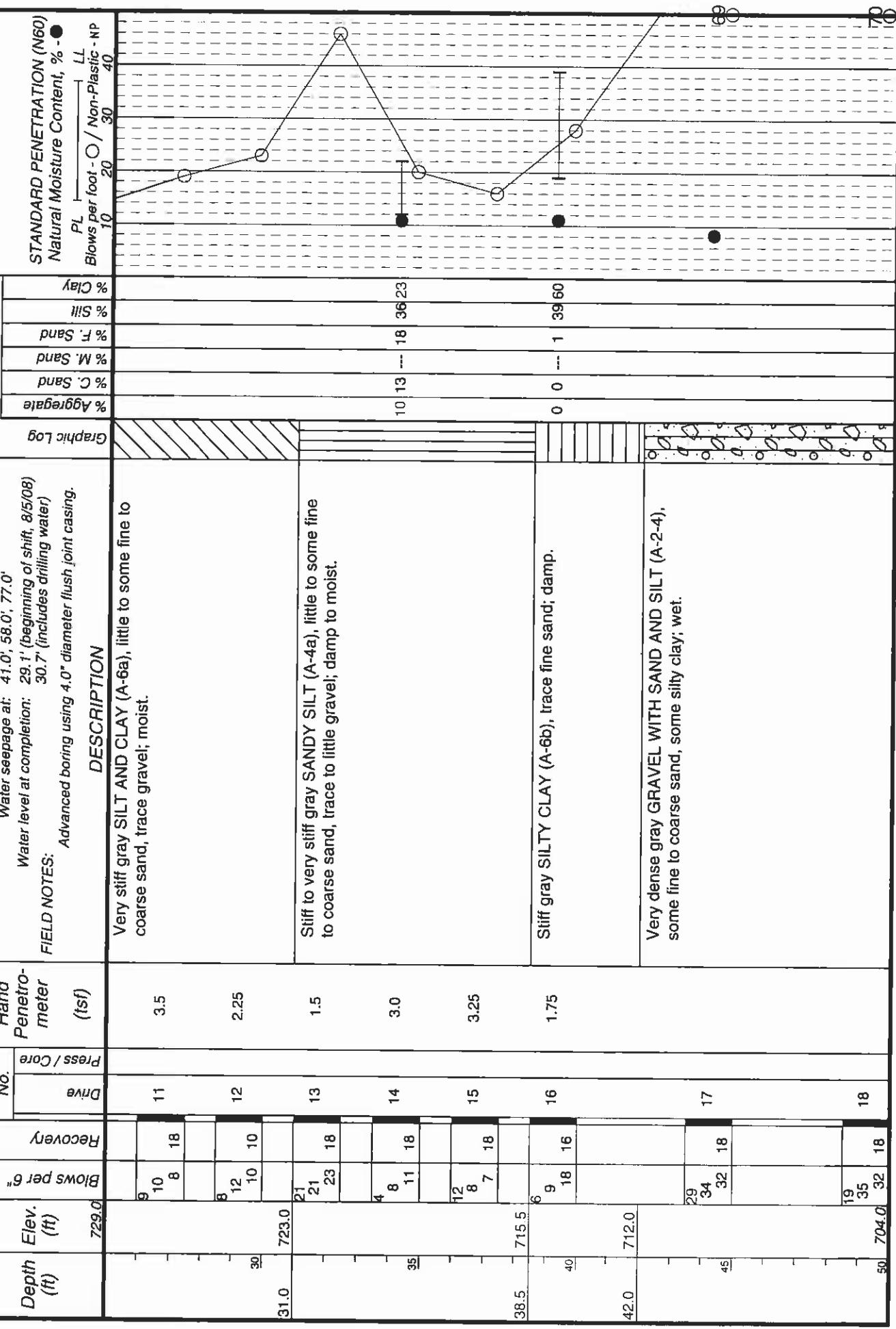
Project: FRA-70-8.93

Job No. 02221-1004.01

LOG OF: Boring B-026

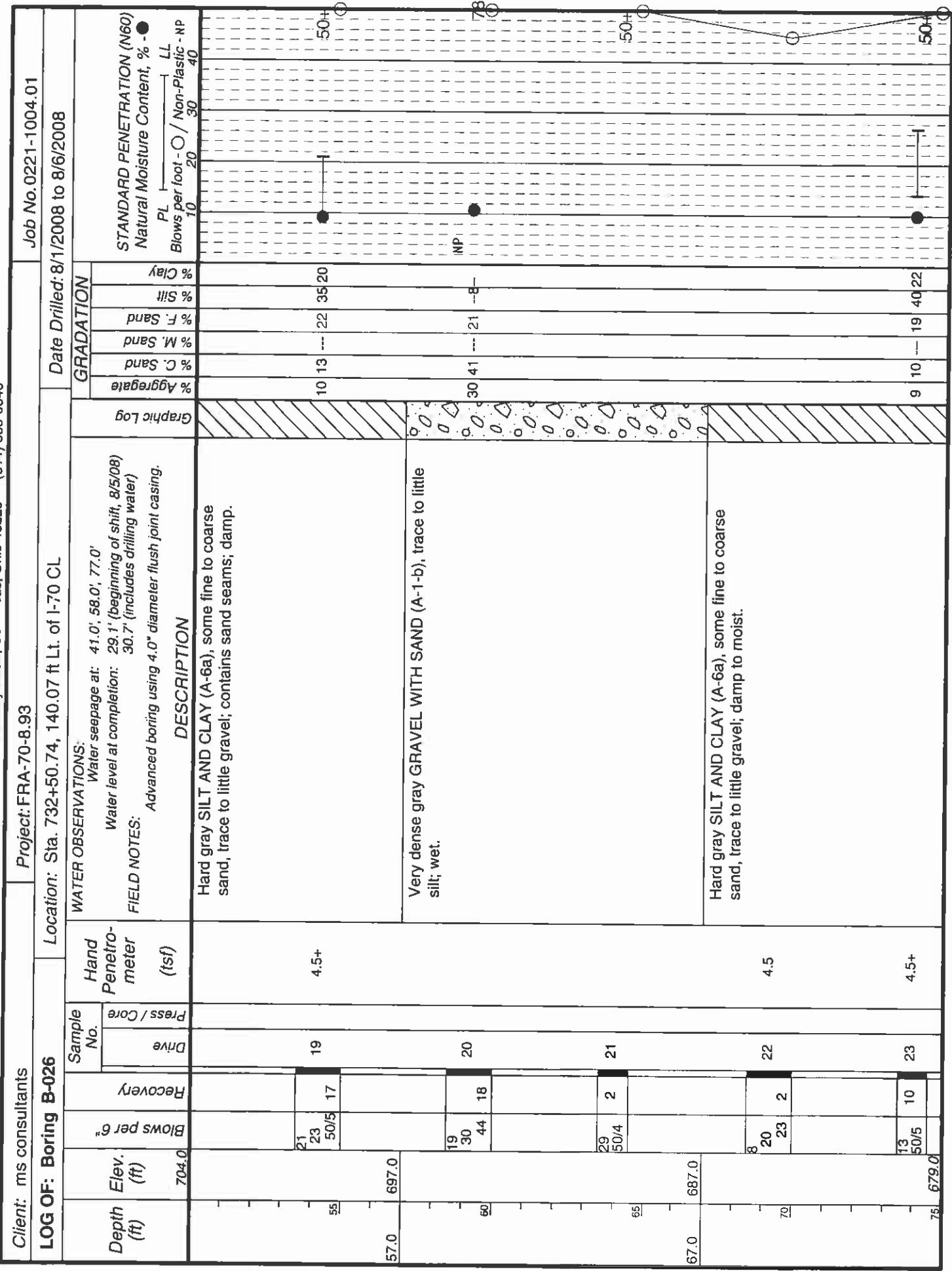
Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL

Date Drilled: 8/1/2008 to 8/6/2008



Client: ms consultants

Project: FRA-70-B.93



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-026

Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL

Depth Elevation (ft)
679.0

Sample No.	Hand Penetrometer (sf)
Press / Core	
Drive	
Recovery	
Blows per 6"	

Water seepage at: 41.0'; 58.0'; 77.0'

Water level at completion: 29.1' (beginning of shift, 8/5/08)

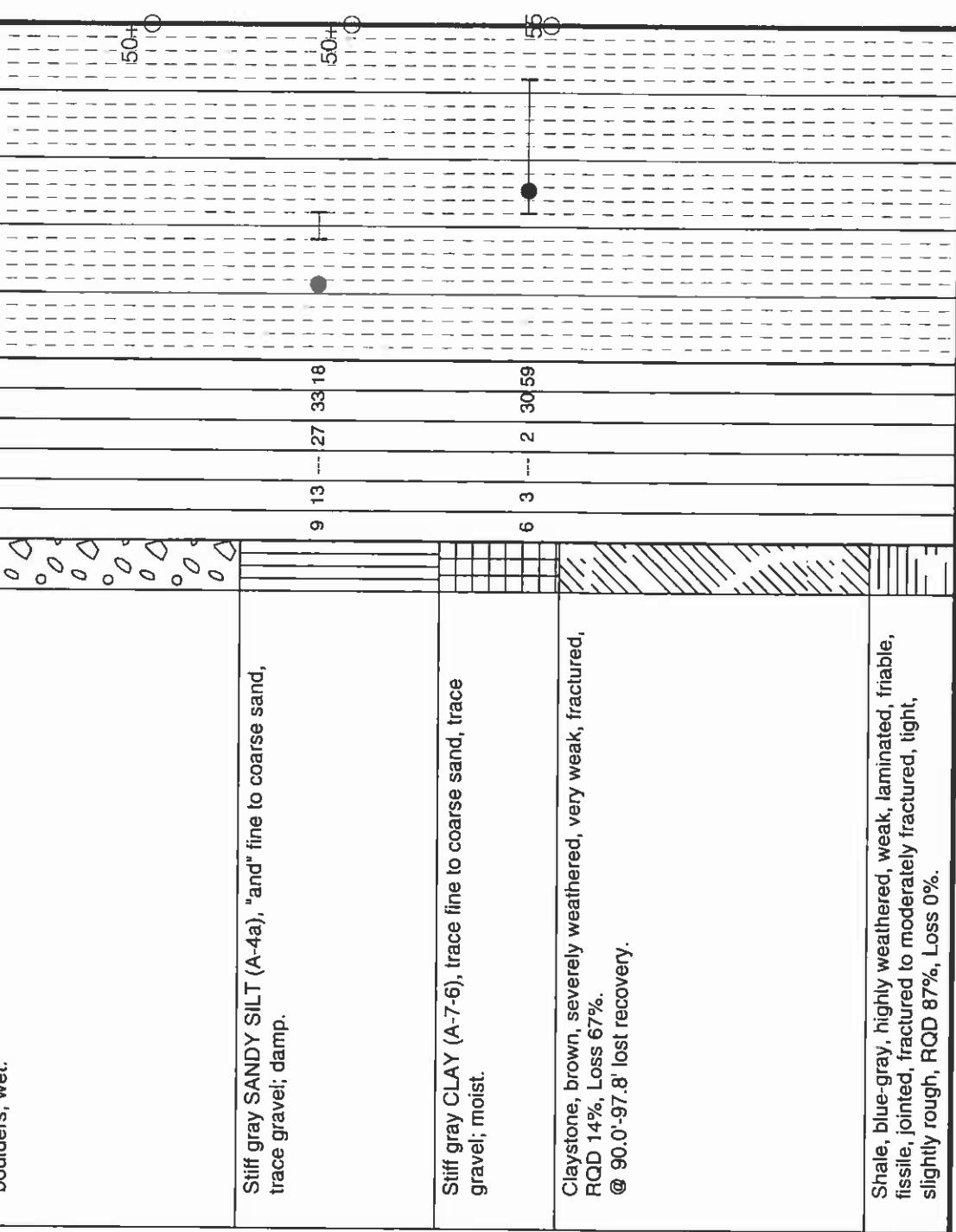
30.7' (includes drilling water)

FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.

DESCRIPTION

Very dense gray GRAVEL (A-1-a); possible cobbles and boulders; wet.

80
82.0 672.0
85
87.0 667.0
90.0 664.0
95
97.8 656.2
100 654.0



Client: ms consultants

Project: FRA-70-8.93

Project: FRA-70-8.93

Lab No 0221 1001 01

Client: ms consultants		Project: FRA-70-8.93						Date Drilled: 8/1/2008 to 8/6/2008		Job No. 00221-1004.01						
LOG OF: Boring B-026		Location: Sta. 732+50.74, 140.07 ft Lt. of I-70 CL														
Depth (ft)	Elev. (ft)	Blows per 6"	Sample No.	Hand Penetrometer (tsf)	Press / Core Drive	WATER OBSERVATIONS:		STANDARD PENETRATION (N60)		% Clay	% Silt	% Sand	% M. Sand	% F. Sand	% Clay	
						Water seepage at: 41.0', 58.0', 77.0'	Water level at completion: 29.1' (beginning of shift, 8/5/08) 30.7' (includes drilling water)	PL	LL							
654.0	654.0	Recovery				FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.		Blows per foot - O / Non-Plastic - NP	Blows per foot - C / Non-Plastic - NP	40	30	30	30	40	40	
105			Core 60"	Rec RQD 85%	R3	DESCRIPTION		Graphic Log								
108.9	645.1		Core 60"	Rec RQD 90%	R4	@ 100.5' - 101.0', qu = 2391 psi										
111.5	639.0		Core 60"	Rec RQD 83%	R5	Interbedded Shale (90%) and Limestone (10%), RQD 88%, Loss 0%; Shale, dark gray, moderately weathered, weak, thinly laminated, calcareous, fissile, pyritic, jointed, moderately fractured, tight, slightly rough; Limestone, light gray, slightly weathered, strong, thinly bedded, pyritic. @ 110.2'-110.5'; 113.9'-114.2'; high angle fractures.									Bottom of Boring - 115.0'	
120																

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 736+63.93, 51.22 ft Rt. of I-70 CL		Date Drilled: 7/8/2008		Job No. 0221-1004.01	
LOG OF: Boring B-027									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS	FIELD NOTES	GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % -		
		Press / Core Drive	Recovery			% Clay	P _L Blows per foot -	LL / Non-Plastic -	
2.4	733.5	16	23	Water seepage at 5.0' Water level at completion. None	Advanced boring using 3.25" diameter hollowstem augers.	% Silt	10	20	
3.0	732.9	17	18		Asphalt Concrete Pavement - 7" Portland Cement Concrete - 11" Aggregate Base - 11"	% Sand	10	20	
5		3	8		FILL: Brick fragments; little fine to coarse sand; damp. Medium dense to dense brown GRAVEL WITH SAND (A-1-b), little to some silt; damp.	% M. Sand	10	20	
		13	12			% C. Sand	10	20	
		9	18			% F. Sand	10	20	
		8	6			% Aggregates	10	20	
		6	16			Graphic Log	10	20	
		9	14				10	20	
		14	28				10	20	
10.0	725.9	13	26		Dense to very dense brown SANDY SILT (A-4a), trace to little gravel; moist.		10	20	
		29	13				10	20	
		9	27				10	20	
14.0	721.9	22	12		Bottom of Boring - 14.0'		10	20	
		15					10	20	

Client: rms consultants		Project: FRA-70-8.93		Location: Sta. 740+11.22, 55.30 ft Lt. of I-70 CL		Date Drilled: 7/24/2008	Job No. 0221-1004.01
LOG OF: Boring B-028		WATER OBSERVATIONS:					
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	Press / Core Drive	Recovery	DESCRIPTION	
15	730.2	8 6 7 8	1 2			FILL: Loose to medium dense brown GRAVEL (A-1-a), little to some fine to coarse sand, little silty clay; moist.	
5		4 5 2 7					
6.0	725.7	10 20 22 16	3			FILL: Dense brown GRAVEL WITH SAND (A-1-b), little silt; contains brick fragments and silty clay seams; damp.	
8.5	723.2	17 49	4			FILL: Very dense gray GRAVEL (A-1-a), trace fine to coarse sand; contains few silty clay seams; damp.	
10.0	721.7	46 12				Bottom of Boring - 10.0'	

Graph Log

STANDARD PENETRATION (N60)
Natural Moisture Content, % - ●
PL → Blows per foot - ○ / Non-Plastic - NP
LL →
10 20 30 40

Client: ms consultants

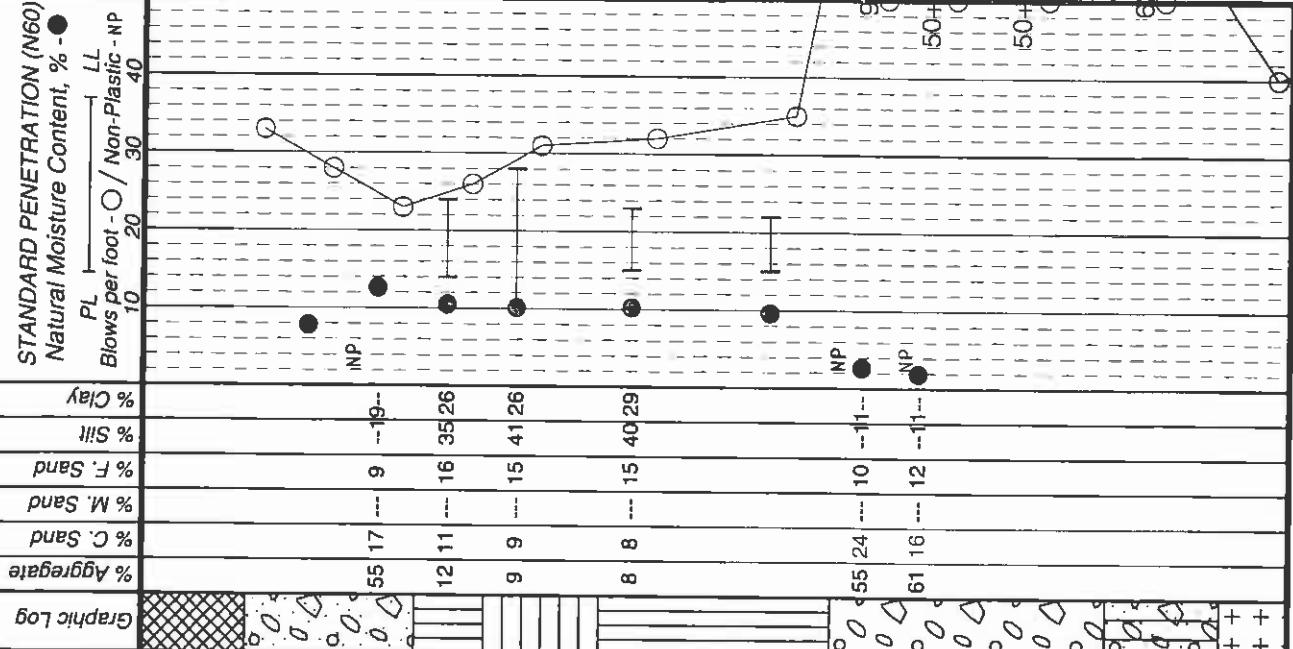
Project: FRA-70-8.93

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Job No. 02221-1004.01

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION		Date Drilled: 7/9/2008 to 7/14/2008			
						Press / Core	Drive			% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay
2.3	740.0	5	15	1					Asphalt Concrete Pavement - 7", Portland Cement Concrete - 9" Aggregate Base - 11"						
		17	7						POSSIBLE FILL: Medium dense to dense brown GRAVEL WITH SAND (A-1-b), little to some silt; damp.						
		13	10	2											
5	736.3	11	13	3											
6.0	736.3	10	6												
7.5	734.8	4	10	4					Hard gray SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.						
		12	15	14					Hard gray SILTY CLAY (A-6b), some fine to coarse sand, trace gravel; damp to moist.						
10.0	732.3	15	15	5											
		15	12												
10.0	732.3	10	14	6					Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.						
		14	17	18					@ 11.5'-21.0', difficult drilling.						
15.0	727.3	8	15	7											
		15	19	18											
21.0	721.3	15	48	8					Very dense gray GRAVEL (A-1-a), some fine to coarse sand, trace to little silt; damp.						
		45	12												
		29	8	9											
		50/3	1												
		50/5	1												
		20													
23.5	718.8	21	21	11					@ 18.5'-18.9', rock fragments; possible cobble blocking shoe.						
		18	21	12					Very dense gray SILT (A-4b), little fine sand; contains some silt; wet.						
25	717.3	11							Dense gray SILT (A-4b), little fine sand; contains interbedded sand seams; wet.						



Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

Job No. 02221-1004.01

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetro- meter (lbf)	WATER OBSERVATIONS:		FIELD NOTES:	DESCRIPTION	GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL - O / Non-Plastic - NP LL - 10 20 30 40
						Press / Core	Drive				
28.5	713.8	717.3	10 19 25	13 13 15	4.5+				Hard gray SANDY SILT (A-4a), "and" fine to coarse sand, trace gravel; contains interbedded sand seams; damp.		
30			13 23 31	14 13 13					Dense to very dense gray COARSE AND FINE SAND (A-3a), some silt; moist.		
31.8	710.5		3 6 29 14 15A 15B		4.5+				Hard gray SANDY SILT (A-4a), some gravel, little to some fine to coarse sand; damp. @ 31.0'-43.5'; difficult drilling. @ 33.5', 5 inches sand heave.		
35			13 26 31	16 9	--						
40											
42.0	700.3										
44.2		698.1		10 20	18A				Dense gray COARSE AND FINE SAND (A-3a), little silt; contains silty clay seams; wet.		
45				29	18	18B					
47.0	695.3								Dense gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; moist.		
50	692.3			19 37 50/3	15				Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet. @ 48.5', 6 inches sand heave.		
										21 48 --- 27 -4- NP	50+ O

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft Rt. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION				STANDARD PENETRATION (N60) Natural Moisture Content, % - ● Blows per foot - ○ / Non-Plastic - NP PL LL 10 20 30 40
				Press/Cone Drive	Recovery			% Aggregate wet.	% C. Sand	% M. Sand	% F. Sand	
55	692.3			10 50/5	16 13		Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; wet.	0.0	0.0	0.0	0.0	50+
57.0	685.3			10 50/5	16 13	20		0.0	0.0	0.0	0.0	50+
60				10 50/5	11 11	21	Very dense gray COARSE AND FINE SAND (A-3a), little to some silt, trace to little gravel; wet. @ 58.5'-68.5', three to six inches sand heave.	0.0	0.0	0.0	0.0	50+
65				10 50/5	11 11	22		0.0	0.0	0.0	0.0	50+
70				10 50/5	13 14	23		0.0	0.0	0.0	0.0	50+
73.8		668.5		10 50/6	25 18	24A 24B		0.0	0.0	0.0	0.0	50+
75		667.3		10 50/6	25 18	24A 24B		0.0	0.0	0.0	0.0	50+
							Very stiff gray SILT (A-4b), little fine sand; moist.	0.0	0.0	0.0	0.0	50+

Project: FRA-70-8.93

LOG OF: Boring B-029

Location: Sta. 740+41.62, 85.96 ft RT. of I-70 CL

Date Drilled: 7/9/2008 to 7/14/2008

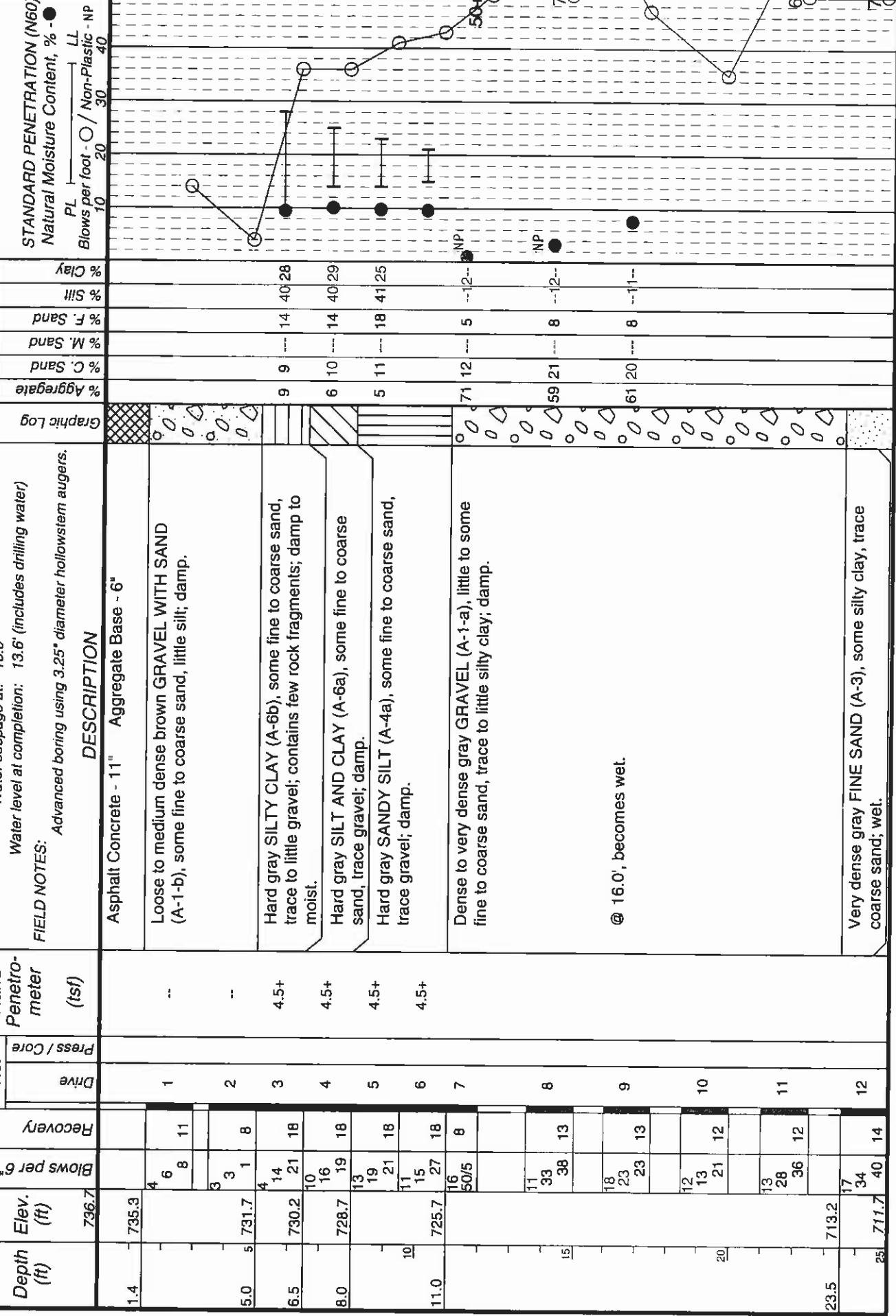
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-030

Location: Sta. 743+00.05, 64.213 ft Lt. of I-70 CL

Job No. 02221-1004.01



Client: ms consultants

Project: FRA-70-B-93

FIELD NOTES:

LOG OF: Boring B-030

Job No. 0221-1004.01

Location: Sta. 743+00.05, 64.213 ft Lt. of I-70 CL

Date Drilled: 7/20/2008 to 7/23/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL → LL Blows per foot - ○ / Non-Plastic - NP 10 20 30 40	
					Press / Core	Drive			% Cbry	% Sili		
55	686.7	50/6	41	10	19							
60				24	39	45	13	20				
62.0	674.7							21				
65				16	31	41	15					
67.0	669.7											
70				33	50/5	9	22					
75	661.7			43	50/3	9	23					

Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-030

Location: Sta. 743+00.05, 64.213 ft Lt. of I-70 CL

Job No. 02221-1004.01

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Press / Core	Drive	Hand Penetro- meter (sf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % - ●	PL Blows per foot - ○ / Non-Plastic - NP LL 30 40 50+ ○			
							Sample No.	Core			% Clay	% Silt	% F. Sand	% M. Sand	% C. Sand	% Aggregate	Graphic Log
661.7	636.7	24								Very dense gray GRAVEL WITH SAND (A-1-b), trace to little silty clay; wet.	0	0	0	0	0	0	0
										② 78.5', 1.0 foot sand heave.	15	39	33	33	13	13	0
										② 83.5', 4.6 feet sand heave.	15	39	33	33	13	13	0
											15	39	33	33	13	13	0
											37	9	27	27	28	28	0
											38	7	28	28	28	28	0
											50/3						0
											636.7						0

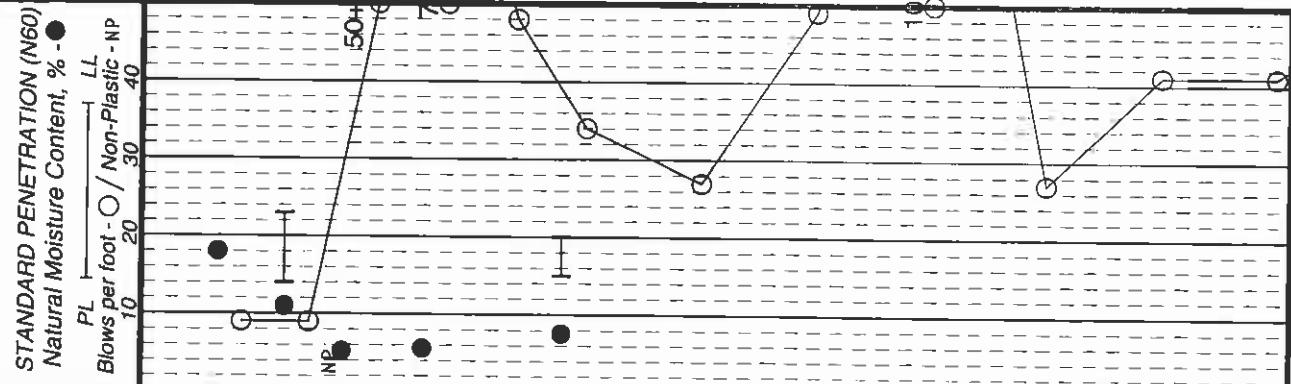
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-031

Location: Sta. 745+02.00, 72.77 ft Rt. of I-70 CL

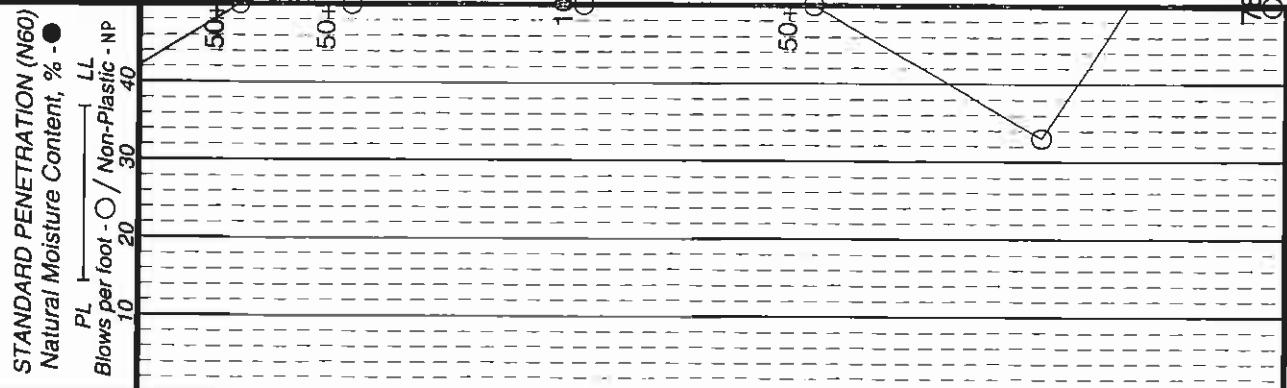
Depth (ft)	Elev. (ft)	Borings per 6' ft)	Recovery	Sample No.	Hand Penetro- meter (tsf)	Press / Core	Drive	WATER OBSERVATIONS:			GRADATION			Date Drilled: 7/7/2008 to 7/8/2008	Job No. 0221-1004.01					
								Water seepage at: Water level at completion:	9.5' 8.3' (includes drilling water)	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	% Aggregate	% C. Sand	% M. Sand	% F. Sand	% Silt	% Clay	Blows per foot - O / Non-Plastic - NP	LL	Natural Moisture Content, %	PL
0.7	734.9	735.6		2	4	1		3.75		Topsoil - 8"										
4.0	731.6			3	5	18				FILL: Very stiff to hard brown SANDY SILT (A-4a), little fine to coarse sand, trace to little gravel; contains few brick and coal fragments; damp.										
8.5	727.1			4	16	5	50/5	5	3	Dense to very dense brownish gray GRAVEL WITH SAND (A-1-b), little silt; damp.	50	20	---	13	--17--					
9.5	726.1			5	42	4				@ 5.0', encountered refusal; offset boring approx. three feet west.	0.0	0.0	0.0	0.0	0.0	0.0				
10				6	31	1				@ 5.5'-8.5', rock fragments; possible cobble blocking shoe.	0.0	0.0	0.0	0.0	0.0	0.0				
13.5	722.1			7	18	6A	6B			Hard gray SANDY SILT (A-4a), trace to little gravel; damp.	15	12	--25	32	16	0				
18.5	717.1			8	12	7				Medium dense to dense gray SILT (A-4b), some fine sand; wet.	+	+	+	+	+	+				
21.0		714.6		9	14	18				Very stiff to hard gray SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.	+	+	+	+	+	+				
23.5		712.1		10	18	18				Medium dense gray SILT (A-4b), trace fine sand; moist.	+	+	+	+	+	+				
25		710.6		11	21	18				Dense gray COARSE AND FINE SAND (A-3a), some silt; wet.	+	+	+	+	+	+				
				12	18	18				Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.	102									



Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-031		Location: Sta. 745+02.00, 72.77 ft Rt. of I-70 CL		Date Drilled: 7/7/2008 to 7/8/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:	
		Recovery	Drive	Water seepage at: 8.3' (includes drilling water)	Water level at completion: 8.3' (includes drilling water)
		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.		DESCRIPTION	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	Press / Core	Drives
28.0	707.6	710.6	Blows per 6"	Recovery	Blows per 6"
				20	10
				50/5	13
				50/5	14
				50/5	5
				24	15
				49	18
				48	18
				50/5	5
				50/5	5
				11	17
				13	18
				16	18
				16	18
				33	18
				41	18
				47.0	688.6
				685.6	693.6
				50	688.6



@ 28.5'-38.9', possible cobbles.

Dense gray FINE SAND (A-3), trace silt; wet.

Very dense gray COARSE AND FINE SAND (A-3a), little silt,
little gravel; wet.
@ 48.5', ten feet sand heave; triconed and washed out.

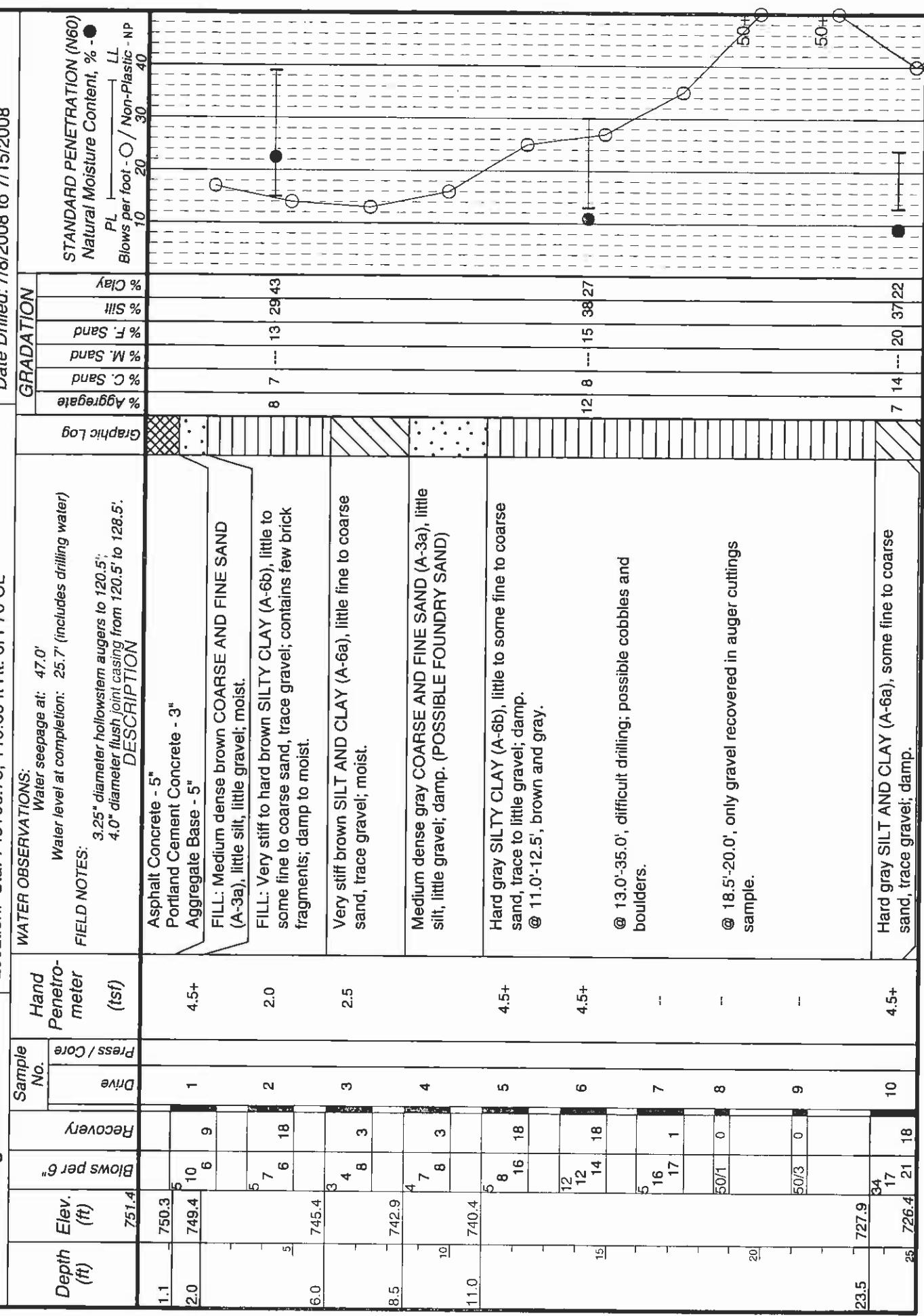
Client: ms consultants		Project: FRA-70-8.93		LOG OF: Boring B-031		Location: Sta. 745+02.00, 72.77 ft Rt. of I-70 CL		Date Drilled: 7/7/2008 to 7/8/2008		Job No. 02221-1004.01		
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION		% Clay	% Silt	STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
				Press / Core	Drive			Blows per 6"	Recovery			
55	685.6			25	33	40	18	19				
60.0	675.6			38	42	49	18	20				
									Bottom of Boring - 60.0'			
										65	70	75

Client: ms consultants

LOG OF: Boring B-032 Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Project: FRA-70-8.93

Job No. 02221-1004.01



Client: ms consultants

Project: FRA-70-8.93

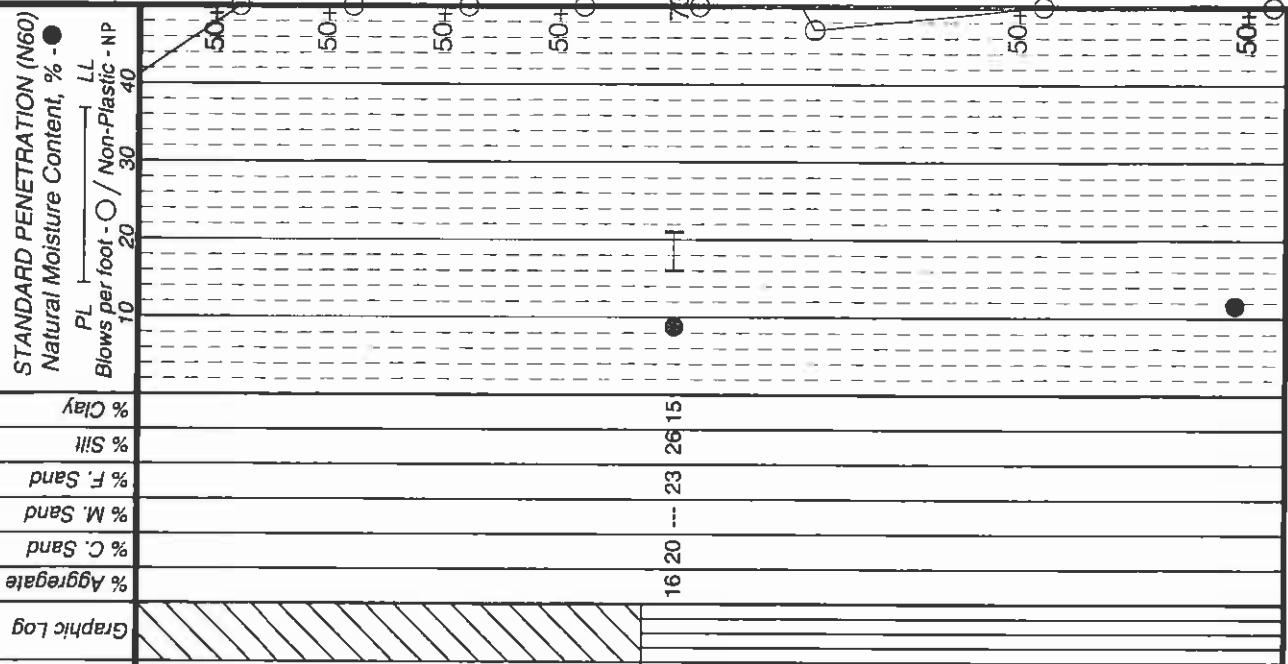
LOG OF: Boring B-032

Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Job No. 02221-1004.01

Depth (ft)	Elev. (ft)	Blows per 6"	Sample No.	Hand Penetro- meter (sf)	WATER OBSERVATIONS:		FIELD NOTES: 3.25" diameter hollowstem augers to 120.5'; 4.0" diameter flush joint casing from 120.5' to 128.5'.	DESCRIPTION: Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; damp.	GRADATION		Date Drilled: 7/8/2008 to 7/15/2008	
					Drive	Press / Core			% Aggreg ate	% C. Sand	% M. Sand	
30	726.4	50/1	1	11	--	--						
30		50/4	0	12	--	--						
35		50/1	0	13	--	--						
35		50/3	1	14	--	--						
36.0	715.4							Hard gray SANDY SILT (A-4a), some to "and" fine to coarse sand, little gravel; damp.				
		10	24	15	4.5+							
		45	45	18								
		11	17	16	4.5+							
		28	28	18								
		11	50/4	10	4.5+							
		32	50/3	9								
		701.4										

④ 48.5'-49.3', contains occasional thin sand seams.



Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL		Date Drilled: 7/8/2008 to 7/15/2008		Job No. 02221-1004.01	
LOG OF: Boring B-032									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS:		GRADATION		% Clay	STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
		Press / Core	Drive	Water seepage at: 47.0' Water level at completion: 25.7' (includes drilling water)	FIELD NOTES: 3.25" diameter hollowstem augers to 120.5'; 4.0" diameter flush joint casing from 120.5' to 128.5'; DESCRIPTION Very dense gray FINE SAND (A-3); contains silty clay seams; wet.	% C.I. % S.I. % F. Sand % M. Sand % C. Sand % Aggreg ate	% Clay % S.I. % F. Sand % M. Sand % C. Sand % Aggreg ate		
55	701.4	Blows per 6"	Recovery	26 50/3	9 19	50+ ○	50+ ○	50+ ○	50+ ○
57.0	694.4			18 50/4	10 20	50+ ○	50+ ○	50+ ○	50+ ○
				28 50/3	9 21	50+ ○	50+ ○	50+ ○	50+ ○
				16 42 50/3	15 22	50+ ○	50+ ○	50+ ○	50+ ○
				33 50/5	11 23	50+ ○	50+ ○	50+ ○	50+ ○
				676.4		50+ ○	50+ ○	50+ ○	50+ ○

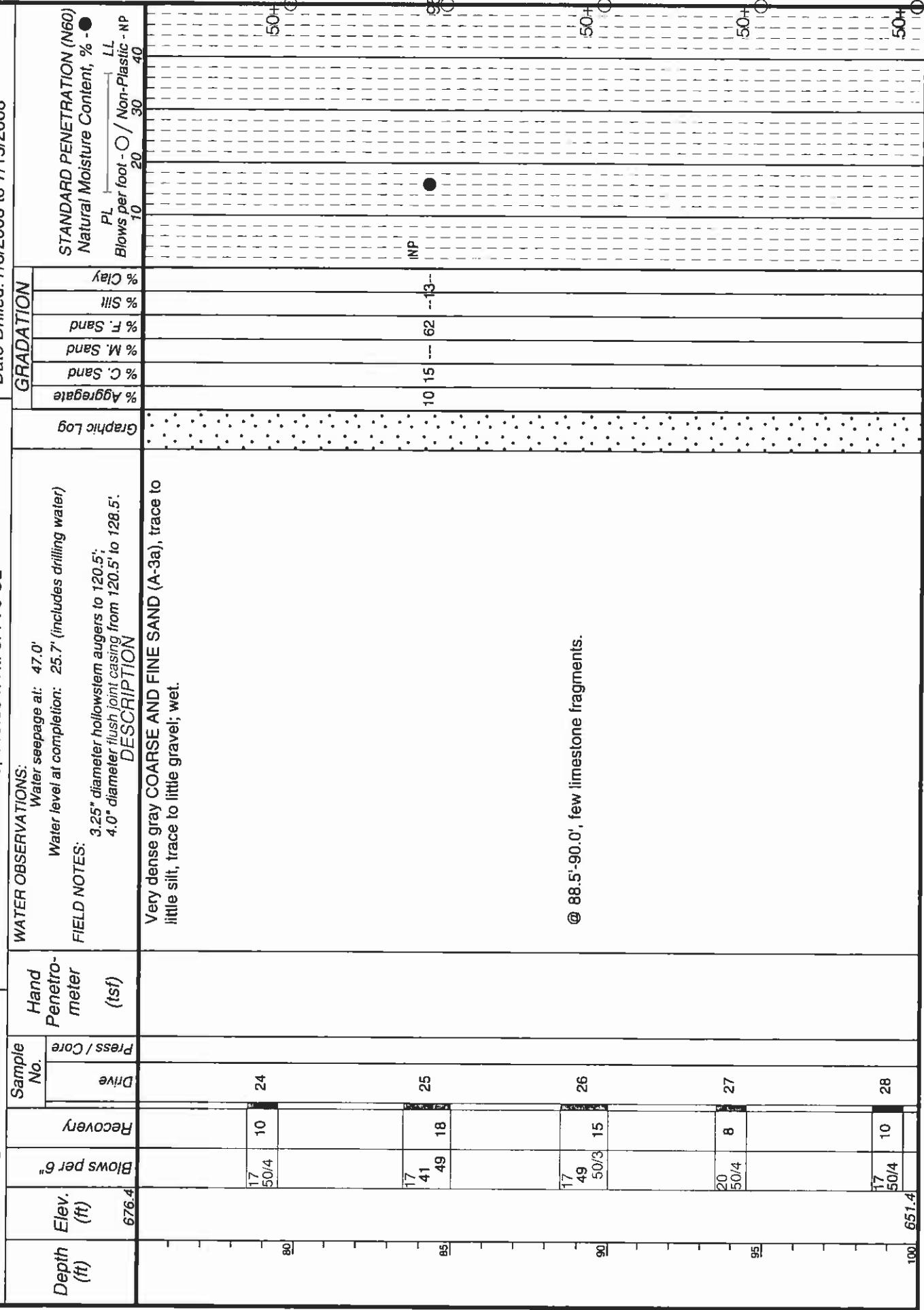
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-032

Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Job No. 0221-1004.01



Client: ms consultants

Project: FRA-70-8.93

Job No. 02221-1004.01

LOG OF: Boring B-032

Location: Sta. 745+06.78, 119.69 ft Rt. of I-70 CL

Date Drilled: 7/8/2008 to 7/15/2008

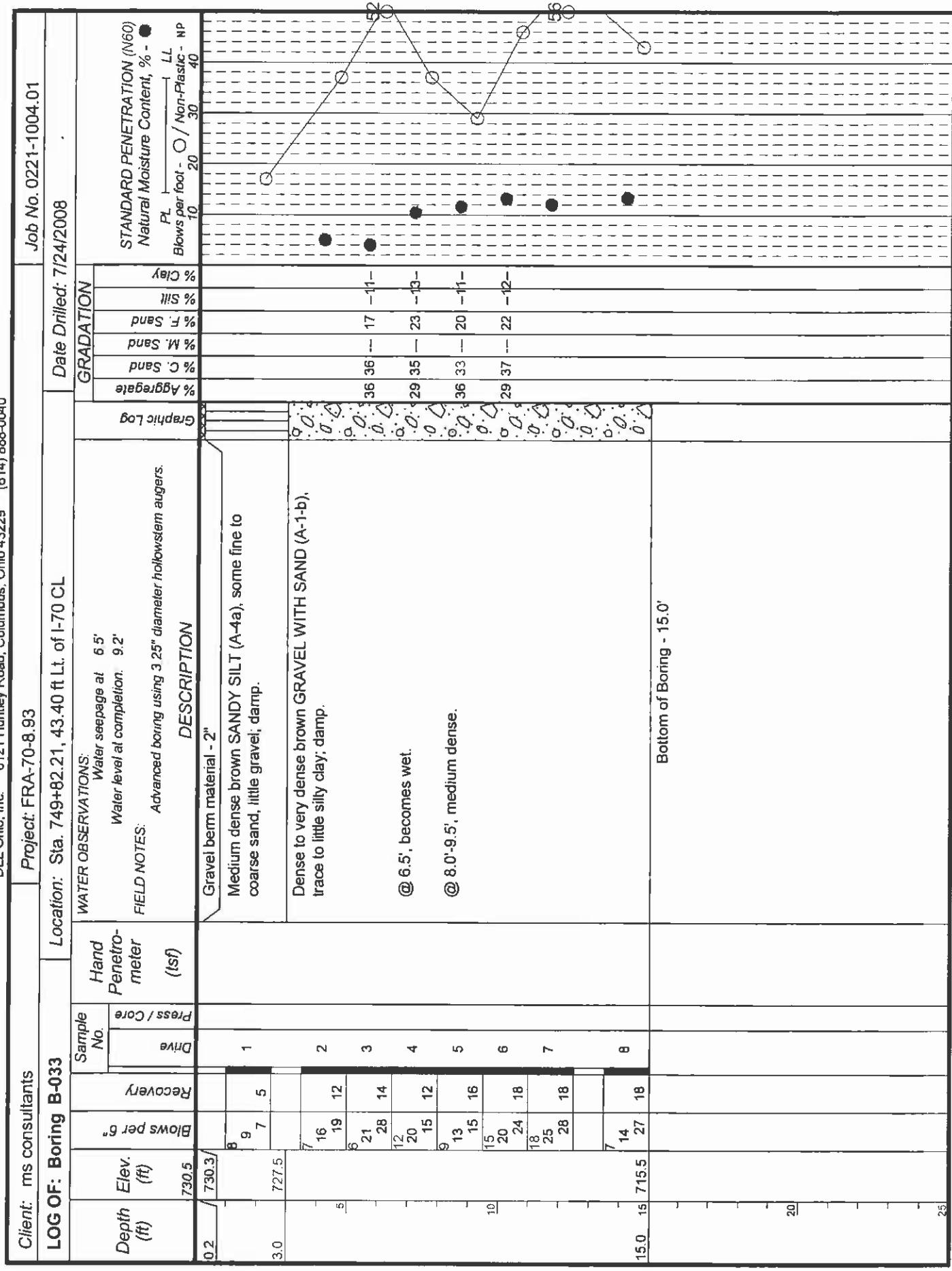
Depth (ft)	Elev. (ft)	Sample No.		Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES:	GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL □ Blows per foot - ○ / Non-Plastic - NP 10 20 30 40 50+ ○					
		Press / Core	Drive		Water seepage at: 47.0' Water level at completion: 25.7' (includes drilling water)	3.25" diameter hollowstem augers to 120.5'; 4.0" diameter flush joint casing from 120.5' to 128.5'; DESCRIPTION		% Clay	% Silt	% Sand	% M. Sand	% C. Sand	% F. Sand	% M. Silt	% S. Silt
105	651.4	Blows per 6"	Recovery												
		537	15												
106		50/3	29												
107		50/4	10												
108		50/4	30												
109		50/5	5												
110															
111															
112															
113															
114															
115															
116															
117.0	634.4														
118		22	10												
119		50/4	32												
120.0	631.4	50/3	33												
120.5	630.9	60*	22*	Core Rec	RQD 8%	R1									
121															
122															
123															
124															
125	626.4														

@ 113.5'-113.9', possible cobbles.

Very dense gray GRAVEL (A-1-a), "and" fine to coarse sand, trace silt; wet.

Severely weathered gray SHALE.

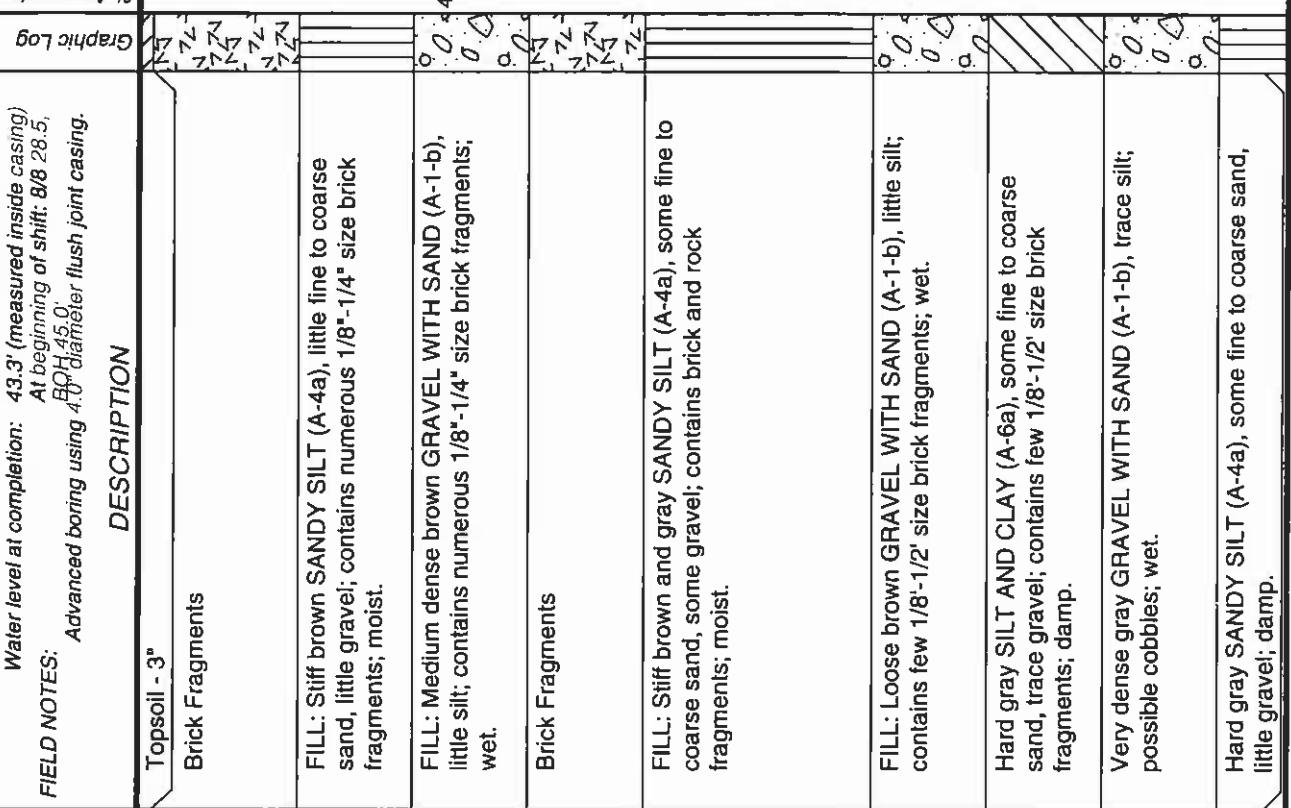
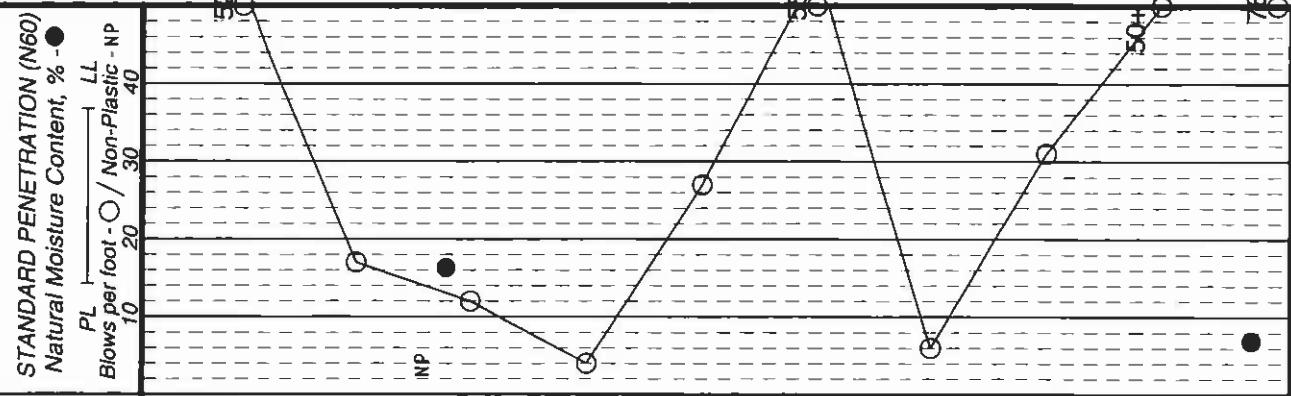
Shale, dark gray, highly to severely weathered, very weak to weak, laminated, calcareous, friable, fissile, pyritic, jointed, fractured to highly fractured, tight, slightly rough; RQD 5%, Loss 53%.



Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-034		Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL		Date Drilled: 8/7/2008 to 8/14/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	GRADATION	
0.3	/ 751.2/	Blows per 6"	Recovery	WATER OBSERVATIONS:	
3.5	748.0	5 7	2	Water seepage at: 5.5'; 10.5'; 20.5'; 25.5'; 62.0' Water level at completion: 43.3' (measured inside casing) At beginning of shift: 8/8 28.5', 45.0' Advanced boring using 4.0" diameter flush joint casing.	
6.0	745.5	2 5	3	DESCRIPTION	
8.5	743.0	5 6	16	FILL: Stiff brown SANDY SILT (A-4a), little fine to coarse sand, little gravel; contains numerous 1/8"-1/4" size brick fragments; moist.	
11.0	740.5	4 6	8	FILL: Medium dense brown GRAVEL WITH SAND (A-1-b), little silt; contains numerous 1/8"-1/4" size brick fragments; wet.	
16.0	735.5	6 2	7	Brick Fragments	
18.5	733.0	4 13	6	FILL: Stiff brown and gray SANDY SILT (A-4a), some fine to coarse sand, some gravel; contains brick and rock fragments; moist.	
21.0	730.5	20 13	13	Brick Fragments	
23.5	728.0	28 45	9	FILL: Loose brown GRAVEL WITH SAND (A-1-b), little silt; contains few 1/8"-1/2" size brick fragments; wet.	
25	726.5	13 23	2	Hard gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; contains few 1/8"-1/2" size brick fragments; damp.	
				Very dense gray GRAVEL WITH SAND (A-1-b), trace silt; possible cobbles; wet.	
				Hard gray SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.	



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
						Press / Core	Drive		
28.5	723.0	30	6 11 14 12	19 5/5 6 12					
31.0	720.5	30	6 4 4 18	13					
33.5	718.0	35	6 19 30 26 18	14 15					
38.5	713.0	40	6 11 26 14	16					
41.0	710.5	40	6 35 41 18	17					
44.0	707.5	45	8 8 9 18	18			<0.25		
47.0	704.5	50	8 28 30 18						
	701.5	50	8 28 50/5	19					

FIELD NOTES:
Medium dense gray GRAVEL (A-1-a), little to some fine to coarse sand, trace silt; contains few brick fragments (likely picked up at higher elevation); wet.

DESCRIPTION

Very dense gray GRAVEL WITH SAND (A-1-b), little silt; wet.

Very loose to loose gray GRAVEL (A-1-a), little fine to coarse sand, trace silt; contains few brick fragments (likely picked up at higher elevation); wet.

Very dense gray GRAVEL WITH SAND (A-1-b), trace to little silt; wet.

@ 36.0'-37.5', contains 3/4"x1-1/2" steel fragment - possible conduit fragment (likely picked up at higher elevation); wet.

Very dense gray SILT (A-4b), trace silt, trace clay; contains occasional sand seams; wet.

Medium dense gray GRAVEL (A-1-a), little fine to coarse sand; wet.

@ 43.5'-44.0', fine sand seam.

Very soft gray SILT AND CLAY (A-6a), some fine to coarse sand, trace gravel; wet.

Very dense gray GRAVEL (A-1-a), some fine to coarse sand; wet.

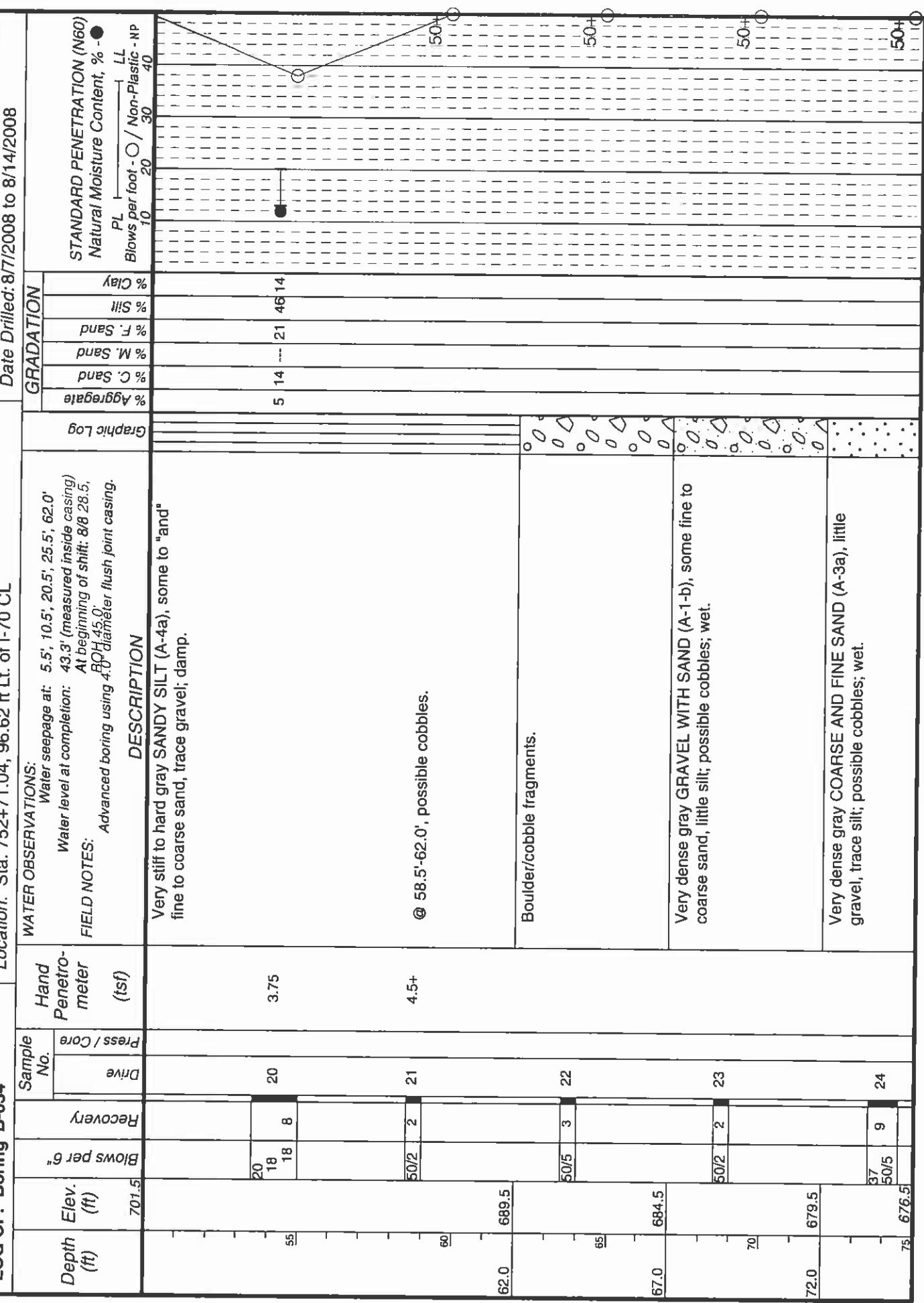
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-034

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Job No. 02221-1004.01



Client: ms consultants

Project: FRA-70-8-93

LOG OF: Boring B-034		Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL		Date Drilled: 8/7/2008 to 8/14/2008		Job No. 0221-1004.01	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS:		GRADATION	
676.5	676.5	Press / Core Drive	Recovery	Water seepage at: 5.5'; 10.5'; 20.5'; 25.5'; 62.0' Water level at completion: 43.3' (measured inside casing) At beginning of shift: 8/8 28.5; Advanced boring using 4 ^{1/2} " diameter flush joint casing.	PL Blows per foot - O / Non-Plastic - LL 10 20 30 40	% Clay % Silt % F. Sand % M. Sand % C. Sand % Aggregate Graphic Log	50+ 0
80		25	25			NP	0
85		38	15			28 31 9 32	0
87.0	664.5	21	26			38	0
87.0	664.5	32	18			50+ 0	0
92.0	659.5	22	27			0	0
92.0	659.5	35	9			0	0
95		50/5	5			0	0
97.0	654.5	35	9			0	0
100	651.5	50/5	5			0	0
						3.25	50+ 0

Client: ms consultants

LOG OF: Boring B-034

Job No. 02221-1004.01

Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL

Date Drilled: 8/7/2008 to 8/14/2008

LOG OF: Boring B-034		Project: FRA-70-8.93		WATER OBSERVATIONS:		FIELD NOTES:		DESCRIPTION		GRADATION		STANDARD PENETRATION (N60)						
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	Press / Core Drive	Recovery	Blows per 6"	Blows per 6"	At beginning of shift: 8/8 28.5, Advanced boring using 40" diameter flush joint casing.	Very dense gray GRAVEL WITH SAND (A-1-b), little to some silt; wet.	% Aggregate	% Clay	% Silt	% Sand	% M. Sand	% C. Sand	% F. Sand	% Non-Plastic - NP	% Natural Moisture Content, % - ●
105	651.5	37	6	30	50/5					25	47	16	--12--					
110		30	33	18	33													
115.0	636.5	23	42	32	50/3	12	33											
115.5	636.0	42	50/3	3	50/3													
120		Core 60*	Rec 39*	RQD 55%	R-1													
125	626.5	Core 60	Rec 53*	RQD 77%	R-2													

Water seepage at: 5.5', 10.5', 20.5', 25.5', 62.0'
 Water level at completion: 43.3' (measured inside casing)
 At beginning of shift: 8/8 28.5,
 Advanced boring using 40" diameter flush joint casing.

DESCRIPTION

Very dense gray GRAVEL WITH SAND (A-1-b), little to some silt; wet.
 @ 100.0'-105.0', possible cobbles.

@ 116.6'-117.0', RQD 69%, Loss 12%. Contains occasional large (up to 1" diameter) pyritic inclusion.

@ 117.5'-119.2', 121.1'-121.7', possible lost recovery.

@ 121.7', moderately fractured, highly weathered.

Severely weathered gray SHALE.

Shale, dark gray, highly to severely weathered, weak, thinly laminated, fissile, calcareous, pyritic, jointed, fractured, light, slightly rough, RQD 69%. Loss 12%. Contains occasional large (up to 1" diameter) pyritic inclusion.

Client: ms consultants		Project: FRA-70-8.93		Date Drilled: 8/7/2008 to 8/14/2008		Job No. 0221-1004.01	
LOG OF: Boring B-034		Location: Sta. 752+71.04, 96.62 ft Lt. of I-70 CL		GRADATION			
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro-meter (tsf)	Press / Core Drive	Blows per 6"	WATER OBSERVATIONS:	STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
130	626.5	Recovery	Core 60*	Rec 60*	RQD 57%	Water seepage at: 5.5', 10.5', 20.5', 25.5', 62.0' Water level at completion: 43.3' (measured inside casing) FIELD NOTES: At beginning of shift: B/QH 28.5, Advanced boring using 4" diameter flush joint casing.	PL 10 Blows per foot - O / Non-Plastic - NP 20 LL 30 40
135	616.0	Core 60*	Rec 60*	RQD 85%	R-4	@ 130.0'-130.3', qu = 3978 psi @ 133.5'-133.7', encountered thin limestone bed.	% Clay % Silt % F. Sand % M. Sand % C. Sand % Aggregate Graphic Log
140						Bottom of Boring - 135.5'	
145							
150							

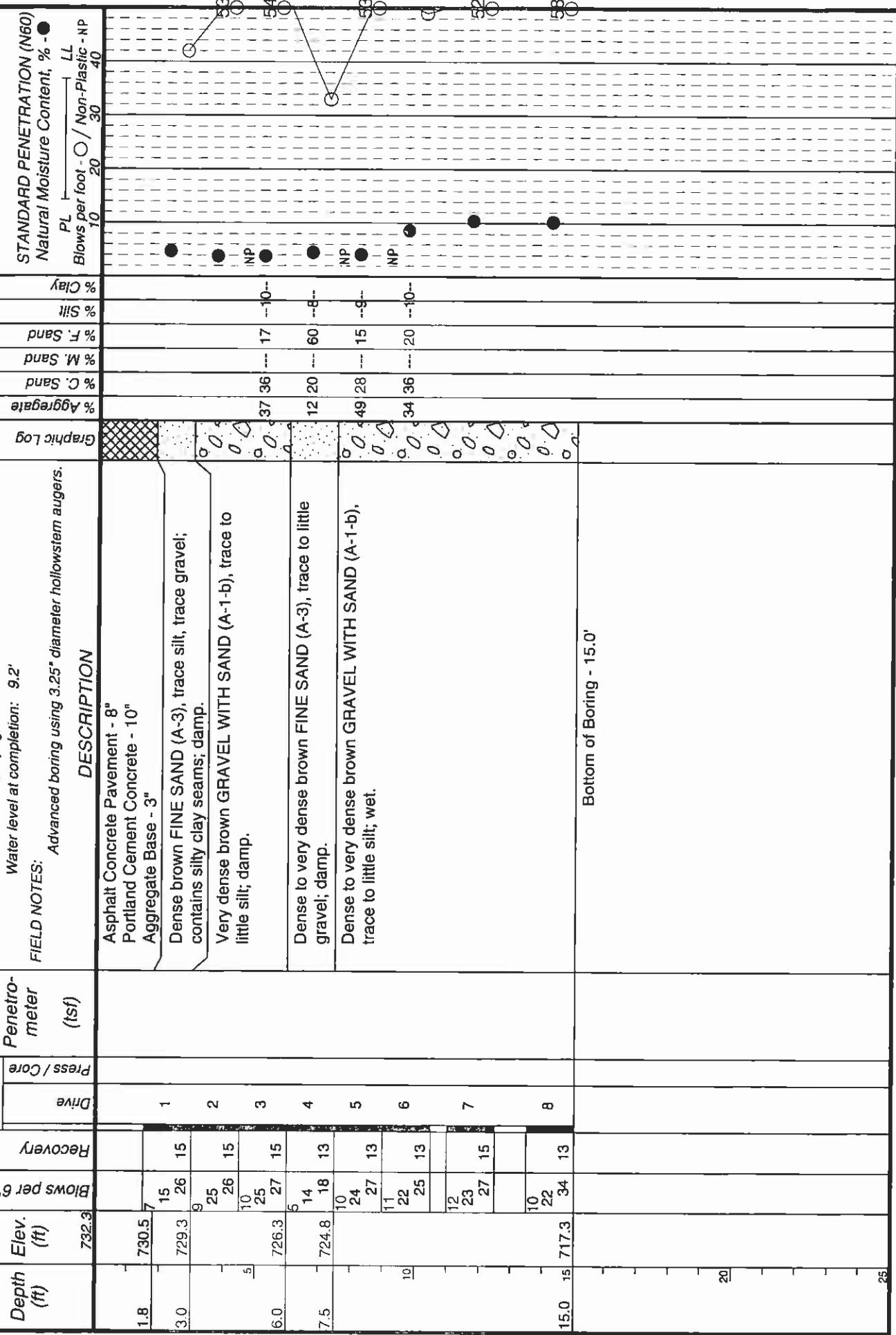
Client: ms consultants

Project: FRA-70-8.93

Job No. 02221-1004.01

LOG OF: Boring B-035

Location: Sta. 752+66.77, 72.74 ft Rt. of I-70 CL



Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-036				Location: Sta. 755+56.04, 101.17 ft Rt. of I-70 CL				Date Drilled: 7/14/2008 to 7/16/2008				Job No. 02221-1004.01								
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:				GRADATION				STANDARD PENETRATION (N60)								
				Drive Blows per 6"	Recovery %	Press / Core Drive	Water seepage at: Water level at completion: (includes drilling water)	% Clay	% Silt	% Sand	% Aggregate	PL Blows per foot	LL Blows per foot	NP Blows per foot	Non-Plastic - NP Blows per foot	40	30	20	10	
28.5	706.2	14	2.5	16	—	15	Very dense brown COARSE AND FINE SAND (A-3a), little silt, trace to little gravel; wet.	NP	—	—	—	5	12	38	42	3	10	23	25	7
30		30	2.5	14	—	17	Stiff to very stiff gray SANDY SILT (A-4a), some to "and" fine to coarse sand, trace to little gravel; moist. @ 29.0'-29.5', sandy silt seam.	NP	—	—	—	10	23	35	7	—	50+	50+	—	—
35		35	—	41	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
40		40	—	42	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
42.0		42.0	692.7	29	10	18	—	—	—	—	—	—	—	—	—	—	—	—	—	—
45		45	—	50/4	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
50		50	684.7	49	14	19	—	—	—	—	—	—	—	—	—	—	—	—	—	—
				39	14	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
				42	14	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
				50/5	12	20	—	—	—	—	—	—	—	—	—	—	—	—	—	—

FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.

DESCRIPTION

Very dense brown COARSE AND FINE SAND (A-3a), little silt, trace to little gravel; wet.

Stiff to very stiff gray SANDY SILT (A-4a), some to "and" fine to coarse sand, trace to little gravel; moist.
@ 29.0'-29.5', sandy silt seam.

@ 33.5'-34.9', contains sand seams.

@ 39.0'-39.3', silt seam.

Very dense gray GRAVEL WITH SAND (A-1-b), trace to little silt; wet.

@ 45.0'-55.0', difficult drilling.

@ 48.5', 2.3 feet sand heave.

Client: ms consultants

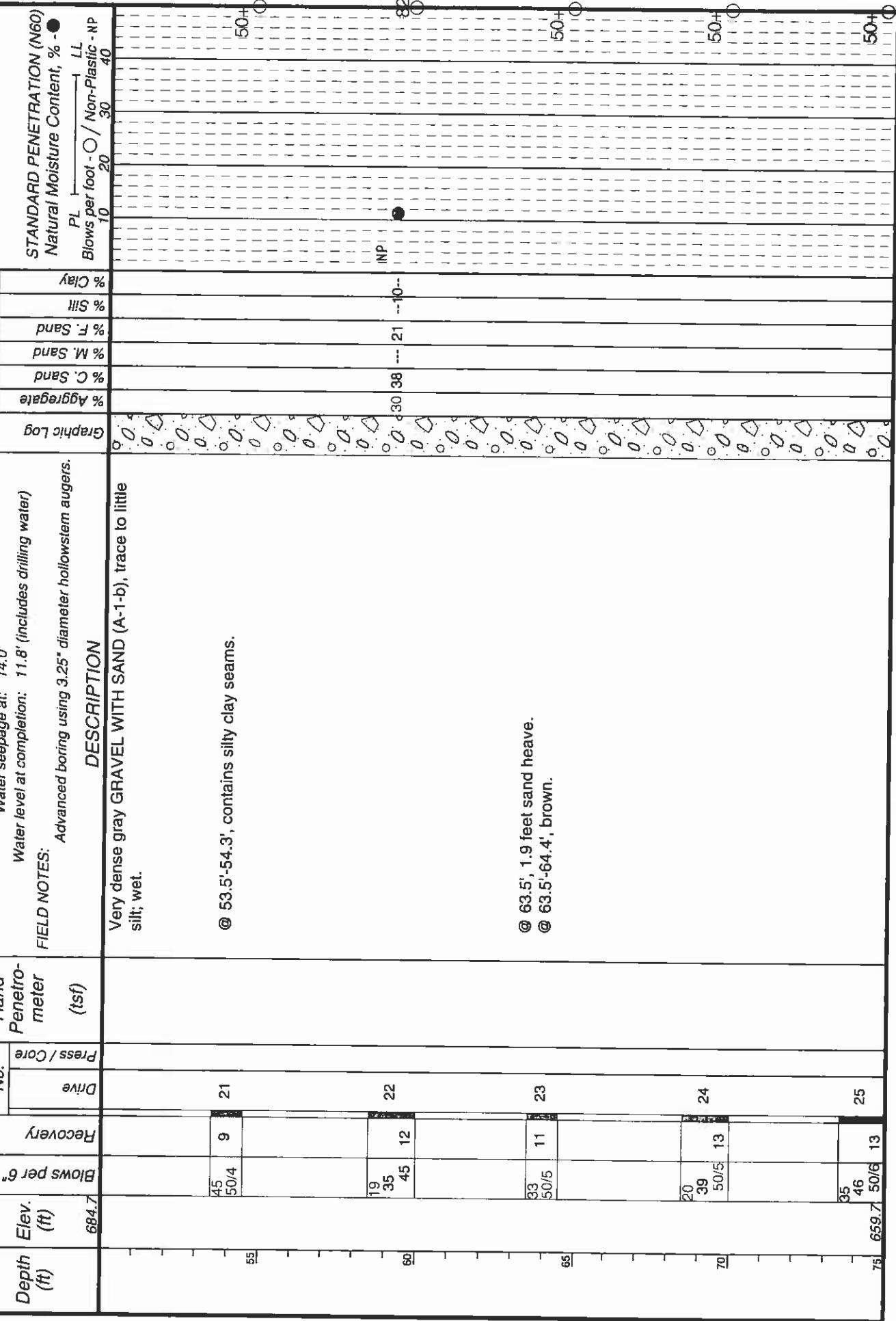
Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-036

Location: Sta. 755+56.04, 101.17 ft Rt. of I-70 CL

Date Drilled: 7/14/2008 to 7/16/2008



Client: ms consultants

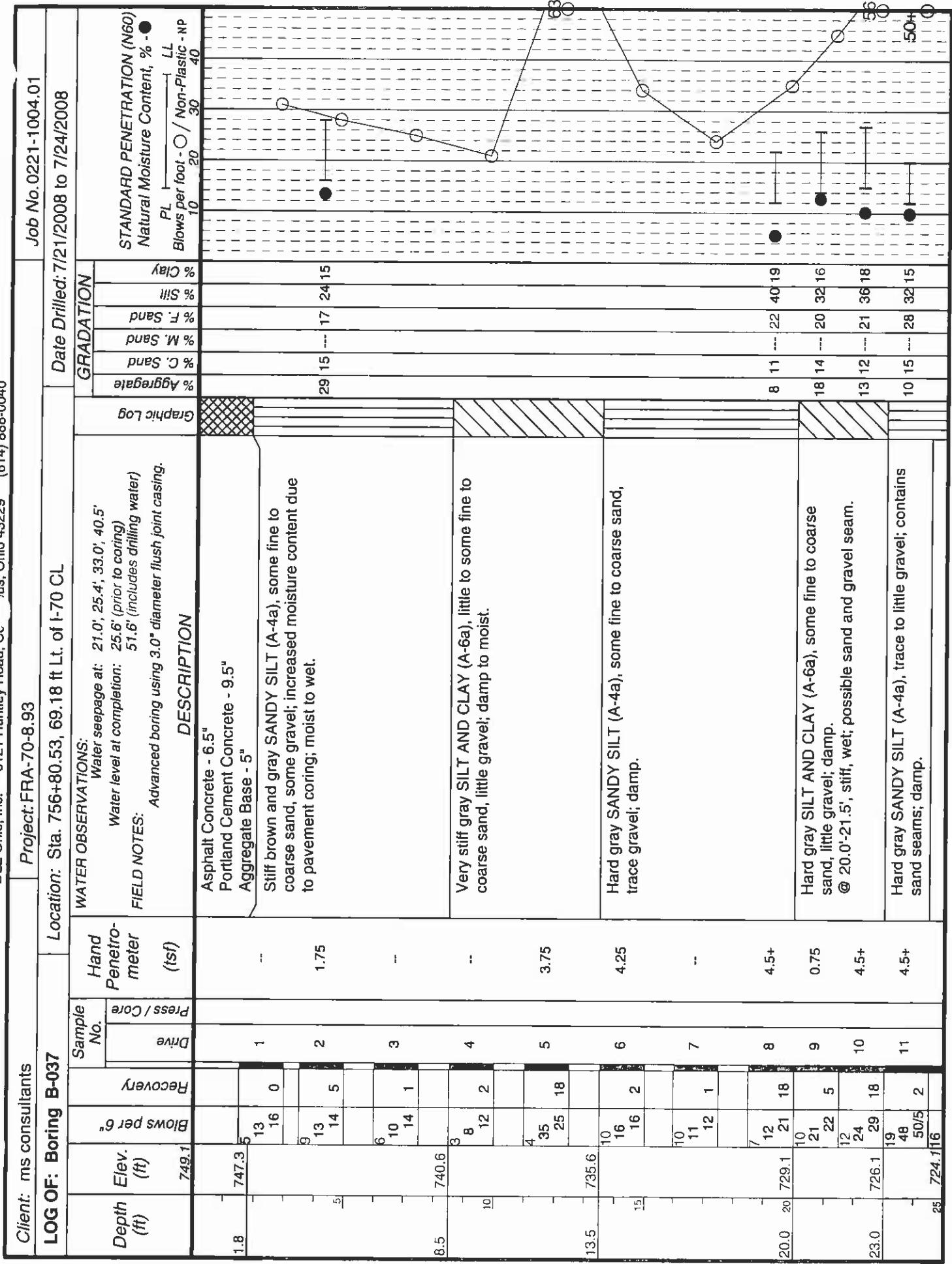
Project: FRA-70-8.93

Location: Sta. 755+56.04, 101.17 ft Rt. of I-70 CL

4/2008 to 7/16/2008

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 755+56.04, 101.17 ft Rt. of I-70 CL		Date Drilled: 7/14/2008 to 7/16/2008		Job No. 00221-1004.01	
LOG OF: Boring B-036									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS:		STANDARD PENETRATION (N60)		INP	●
		Recovery	Drive	Blows per 6"	Press / Core	PL	LL		
93.5	641.2	27	26	50/3	8	50+	50+		
94.9	639.8	50/3	8	27	11	50+	50+		
96.6	638.1	50/3	0	28					
99.0	634.7	Core 68"	Rec 68"	RQD 93%	R1				

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 755+56.04, 101.17 ft Rt. of I-70 CL		Date Drilled: 7/14/2008 to 7/16/2008		Job No. 0221-1004.01	
LOG OF: Boring B-036									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS:		GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % - ●	
		Drive	Press / Core	Water seepage at: 14.0' Water level at completion: 11.8' (includes drilling water)		% Clay % Silt % F. Sand % M. Sand % C. Sand % Aggregate Graphic Log		Blows per foot - ○ / Non-Plastic - NP PL LL 20 30 40	
634.7	634.7	Recovery	Blows per 6"	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.		DESCRIPTION			
105		Core 60"	Rec RQD 83%	Interbedded Shale (90%) and Limestone (10%) RQD 74% Loss 0%; Shale, dark gray, highly weathered, weak, laminated, friable, fissile, pyritic, jointed, moderately fractured, tight, slightly rough; Limestone, gray, moderately weathered, strong, thinly bedded, pyritic, slightly rough. @ 103.7' - 104.2', qu = 1442 psi					
110		Core 60"	Rec RQD 60%						
115	115.6	Core 60"	Rec RQD 68%						
120				Bottom of Boring - 115.6'					
125									



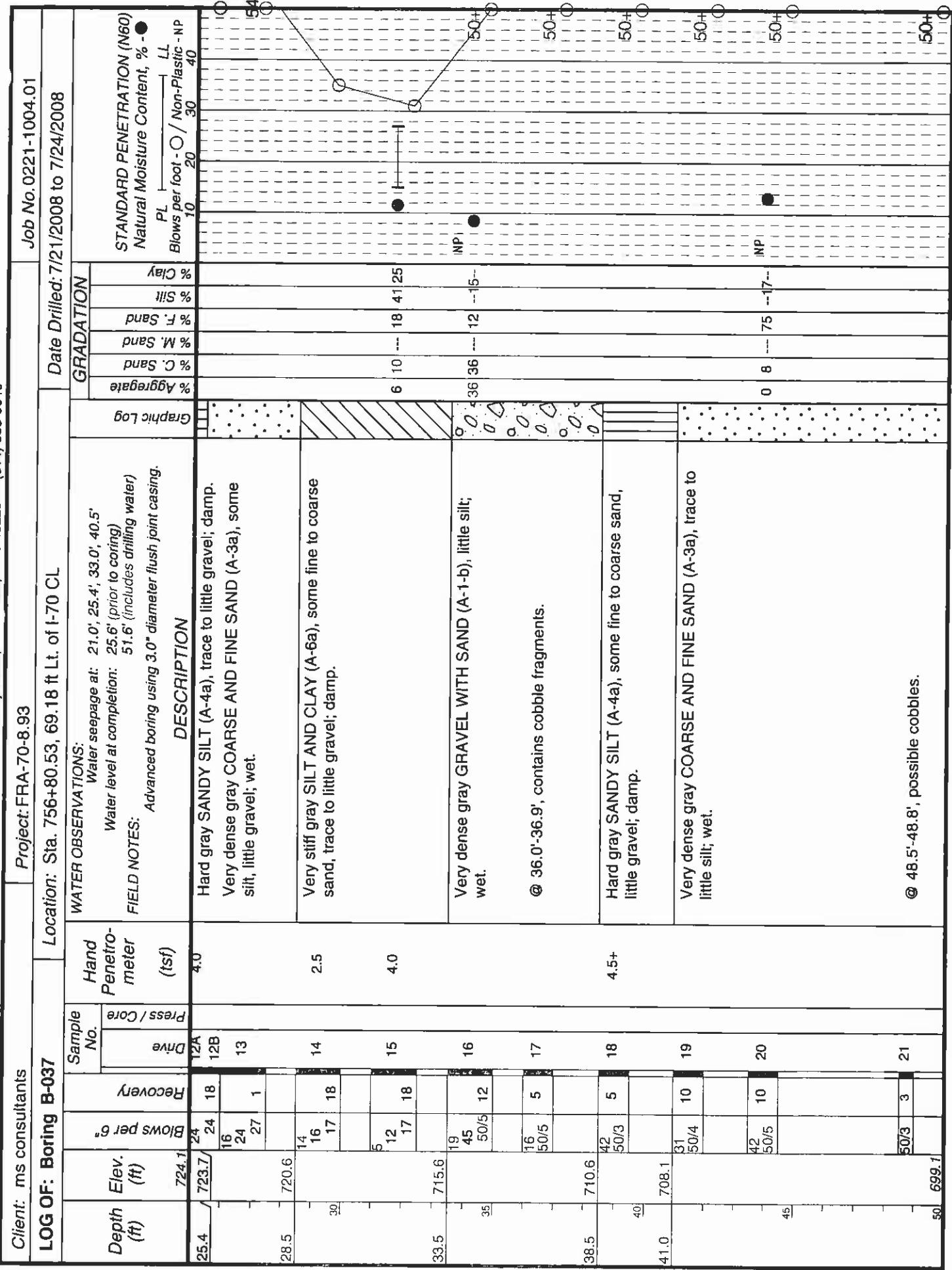
Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-037

Location: Sta. 756+80.53, 69.18 ft Lt. of I-70 CL

Job No. 0221-1004.01



Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 756+80.53, 69.18 ft Lt. of I-70 CL		Date Drilled: 7/21/2008 to 7/24/2008		Job No. 0221-1004.01	
LOG OF: Boring B-037									
Depth (ft)	Elev. (ft)	Blows per 6"	Sample No.	Hand Penetro- meter (ft)	WATER OBSERVATIONS:		GRADATION		
					Water seepage at: 21.0', 25.4'; 33.0', 40.5' Water level at completion: 25.6' (prior to coring) 51.6' (includes drilling water)		STANDARD PENETRATION (N60) Natural/Moisture Content, % - ●	PL Blows per foot - ○ / LL 20 30 40	
55	699.1	Recovery	Drive	Press / Core	FIELD NOTES: Advanced boring using 3.0" diameter flush joint casing.	% Clay	% Silt	% Sand	
57.0	692.1	29 50/5	11	22	4.5+	3	3 --- 10	63 21	
60		25 50/3	3	23		0.0	0.0	0.0	
62.0	687.1				Very dense gray GRAVEL WITH SAND (A-1-b), some fine to coarse sand, trace silt; wet. @ 59.0'-75.0', encountered cobbles and boulders.	0.0	0.0	0.0	
65		45 50/5	4	24		0.0	0.0	0.0	
68		50/3	3	25		17	24 --- 39	-20-	
70		50/4	1	26		NP	50+	0	
75	674.1								50+

@ 73.5'-73.8', drove sampler on gravel; poor recovery.

Client: ms consultants

Project: FRA-70-8.93

Job No. 02221-1004.01

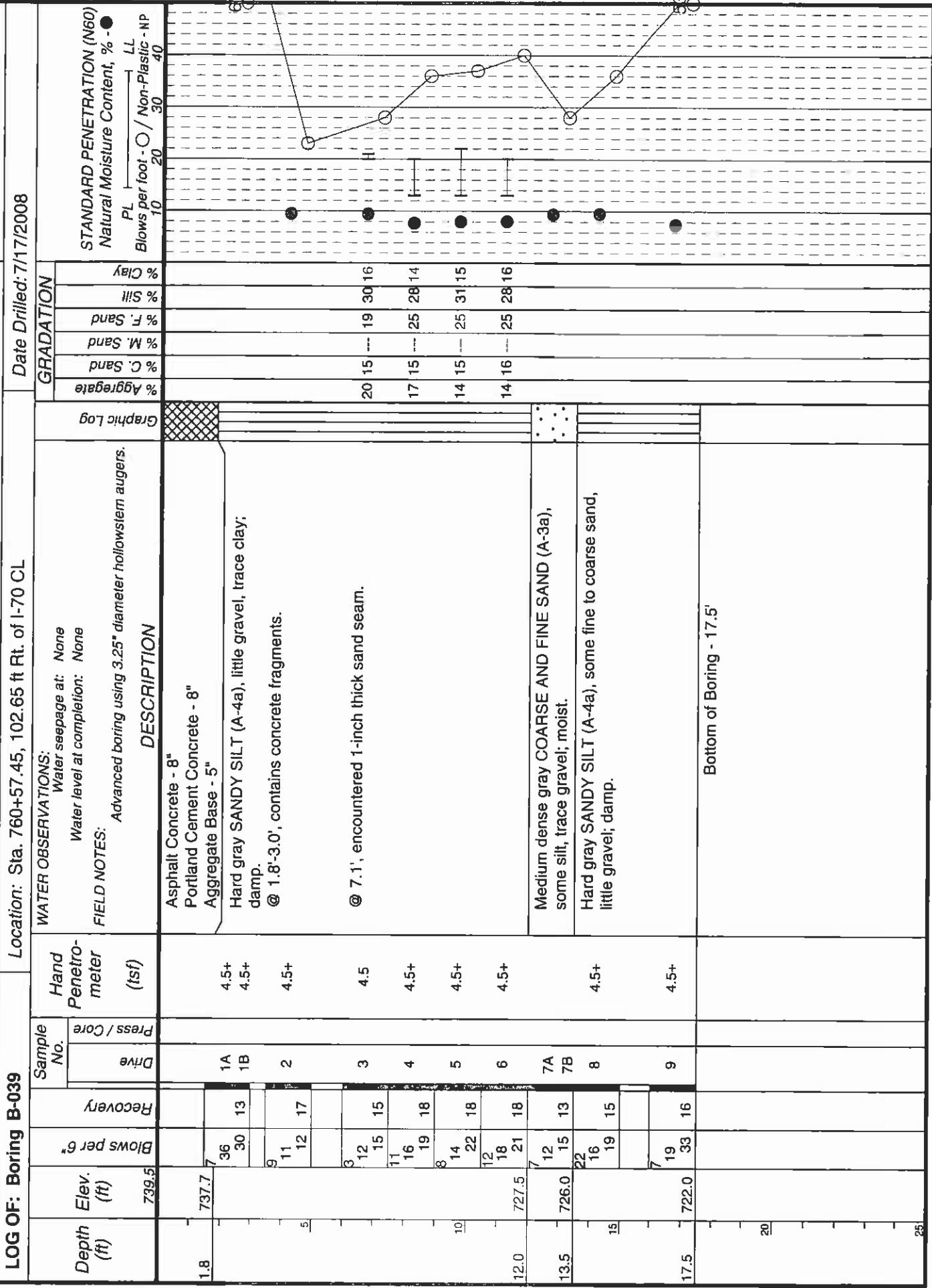
LOG OF: Boring B-037		Location: Sta. 756+80.53, 69.18 ft Lt. of I-70 CL		Date Drilled: 7/21/2008 to 7/24/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (tsf)	WATER OBSERVATIONS:	
649.1	649.1	Press / Core Drive	Recovery	Water seepage at: 21.0', 25.4'; 33.0', 40.5' Water level at completion: 25.6' (prior to coring) 51.6' (includes drilling water) Advanced boring using 3.0" diameter flush joint casing.	
105		23	32	2.75	
108.5	640.6	50/5	5	33	4.5+
110.0	639.1				Severely weathered gray SHALE.
		Core 60"	Rec 60"	RQD 60%	Interbedded Shale (92%) and Limestone (8%) RQD 76% LOSS 0%; Shale, dark gray, highly weathered, weak, laminated, calcareous, moderately fractured to fractured; Limestone, light gray, moderately weathered, moderately strong to strong, thinly bedded.
		Core 60"	Rec 60"	RQD 83%	@ 110.8'-111.2', 111.7'-111.8', 113.2'-113.5', 119.6'-120.0', @ 122.9'-123.0', high angle fractures.
		Core 60"	Rec 60"	RQD 77%	@ 121.2' - 121.7', qu = 1546 psi @ 122.1'-125.4', highly to severely weathered.
					624.1

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 756+80.53, 69.18 ft Lt. of I-70 CL		Date Drilled: 7/21/2008 to 7/24/2008		Job No. 02221-1004.01		
LOG OF: Boring B-037										
Depth (ft)	Elev. (ft)	Blows per 6"	Recovery	Sample No.	Hand Penetro- meter (sf)	WATER OBSERVATIONS:		GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % - ●	
						Water seepage at: 21.0'; 25.4'; 33.0'; 40.5' Water level at completion: 25.6' (prior to coring) 51.6' (includes drilling water)	Advanced boring using 3.0" diameter flush joint casing.			
624.1	624.1	82%	Core 60°	Press / Core R4	Drive RQD 82%	DESCRIPTION		Graphic Log		
130.0	619.1					Interbedded Shale (92%) and Limestone (8%) RQD 76% LOSS 0%; Shale, dark gray, highly weathered, weak, laminated, calcareous, moderately fractured to fractured; Limestone, light gray, moderately weathered, moderately strong to strong, thinly bedded. @ 125.9'-126.0', high angle fracture.				
						Bottom of Boring - 130.0'				

Client: ms consultants

LOG OF: Boring B-039

Job No. 02221-1004.01



LOG OF: Boring B-041		Project: FRA-70-8.93		Location: Sta. 762+58.90, 115.94 ft Lt. of I-70 CL		Date Drilled: 6/23/2008 to 6/25/2008		Job No. 0221-1004.01	
Client: ms consultants									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (fsf)	Recovery	Press / Core Drive	WATER OBSERVATIONS:	GRADATION	STANDARD PENETRATION (N60)	
1.5	758.2	10 8 4 6	1	1		Water seepage at: 13.0'-15.5'; 52.0'-92.0' Water level at completion: 37.1' (includes drilling water) FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.	% Aggregate	PL Blows per foot - O / Non-Plastic - NP LL Blows per foot - O / Non-Plastic - NP 30 40	
4.5	755.2	14 2	2	3	3.0	Medium dense brown GRAVEL (A-1-a), trace to little fine to coarse sand, trace to little silty clay; damp to moist. @ 3.2', encountered large rock fragments.	% C. Sand	% Clay	
5		5 7 8 18		3	3.0	Very stiff to hard brown SANDY SILT (A-4a), some fine to coarse sand, little gravel; damp.	% M. Sand	% Silt	
10		6 9 14 11 15 13	4	5	4.5+	@ 8.5', becomes gray.	% F. Sand	% Silty	
13.0	746.7	12 15 18	6	6	4.5+		% C. Clay	% Clay	
15.5	744.2	17 38 31 18	7	7	4.5+	Very dense brown and gray GRAVEL WITH SAND (A-1-b), some silt; wet.	% M. Clay	% Silty	
25	734.7	10 16 18 12 20 15 23 18 8 25 34 6	8 9 9 10 11	8 9 10 11	4.5+	Hard gray SANDY SILT (A-4a), little gravel; damp to moist.	% F. Clay	% Silty	

Graphic Log

Client: ms consultants

Project: FRA-70-8.93

WATER OBSERVATIONS:

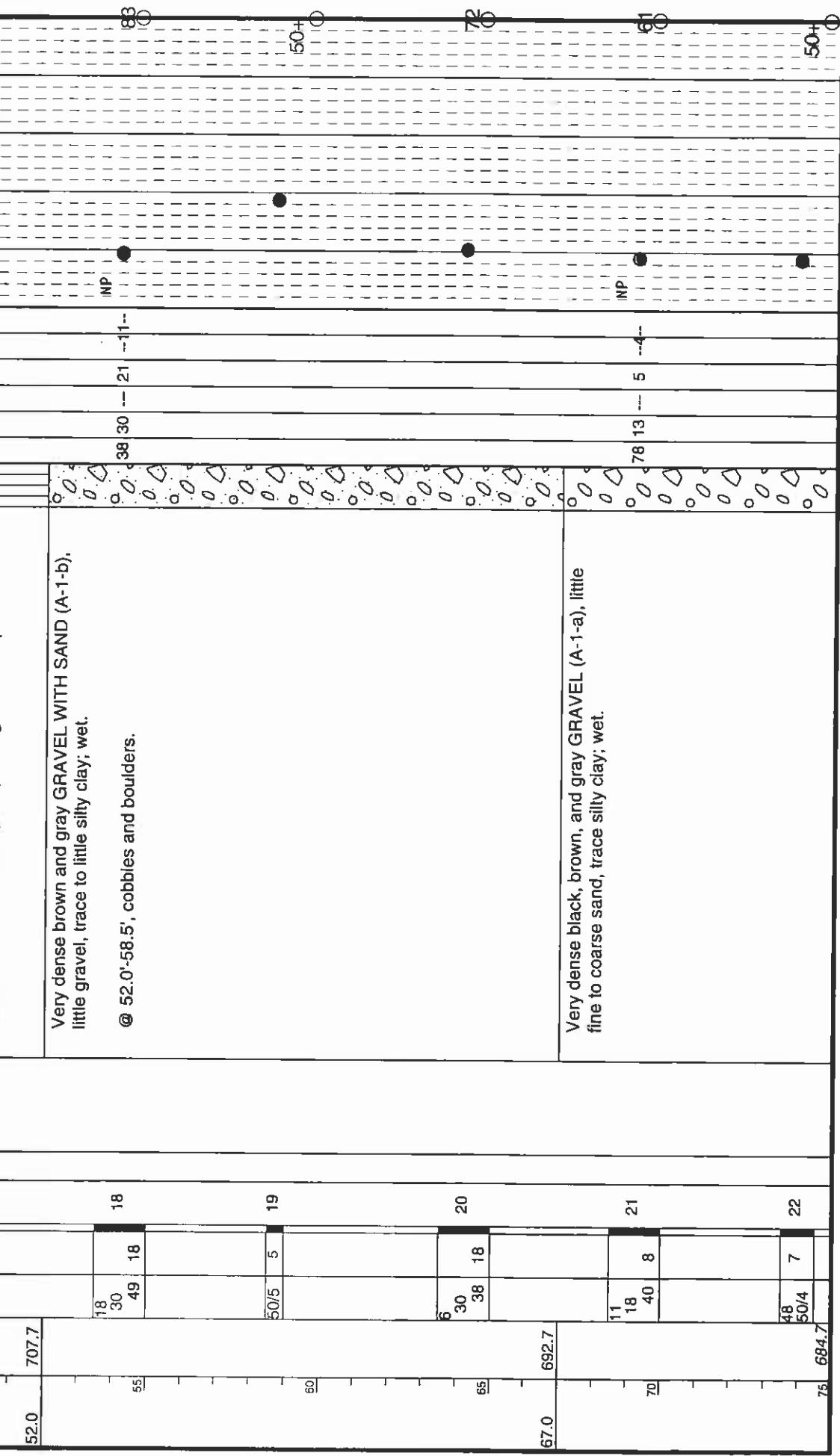
Water seepage at: 13.0'-15.5'; 52.0'-92.0'
Water level at completion: 37.1' (includes drilling water)

FIELD NOTES:

Advanced boring using 4.0" diameter flush joint casing.

DESCRIPTION

Hard gray SANDY SILT (A-4a); little gravel; damp to moist.



Client: ms consultants

Project: FRA-70-8.93

Job No. 02221-1004.01

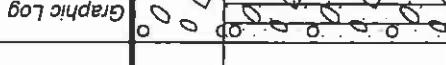
LOG OF: Boring B-041

Location: Sta. 762+58.90, 115.94 ft Lt. of I-70 CL

Date Drilled: 6/23/2008 to 6/25/2008

LOG OF: Boring B-041		Project: FRA-70-8.93		WATER OBSERVATIONS:		GRADATION		STANDARD PENETRATION (N60)	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (sf)	Press / Core Drive	Recovery	FIELD NOTES:	% Clay	% Silt	Natural Moisture Content, % -
77.0	682.7	618	33	23	4	Very dense black, brown, and gray GRAVEL (A-1-a), little fine to coarse sand, trace silty clay; wet.	0	0	PL 10
80		50/5	3	24		Very dense brown and gray GRAVEL WITH SAND AND SILT (A-2-4); wet.	0	0	LL 20
82.0	677.7	37/5	11	25		Very dense gray COARSE AND FINE SAND (A-3a), some silty clay, trace gravel; possible cobbles; wet.	50+	0	Non-Plastic - NP 30
85		50/5	5	26			50+	0	40
90		33	11	27			50+	0	
92.0	667.7	50/5	5			Very dense brownish gray SANDY SILT (A-4a), trace gravel; damp.	9	8	NP 21 42 20
100	659.7	33	11				50+	0	50+

Graphic Log

**DESCRIPTION**

Advanced boring using 4.0" diameter flush joint casing.
Water seepage at: 13.0'-15.5'; 52.0'-92.0'
Water level at completion: 37.1' (includes drilling water)

Very dense brown and gray GRAVEL WITH SAND AND SILT (A-2-4); wet.

Very dense gray COARSE AND FINE SAND (A-3a), some silty clay, trace gravel; possible cobbles; wet.

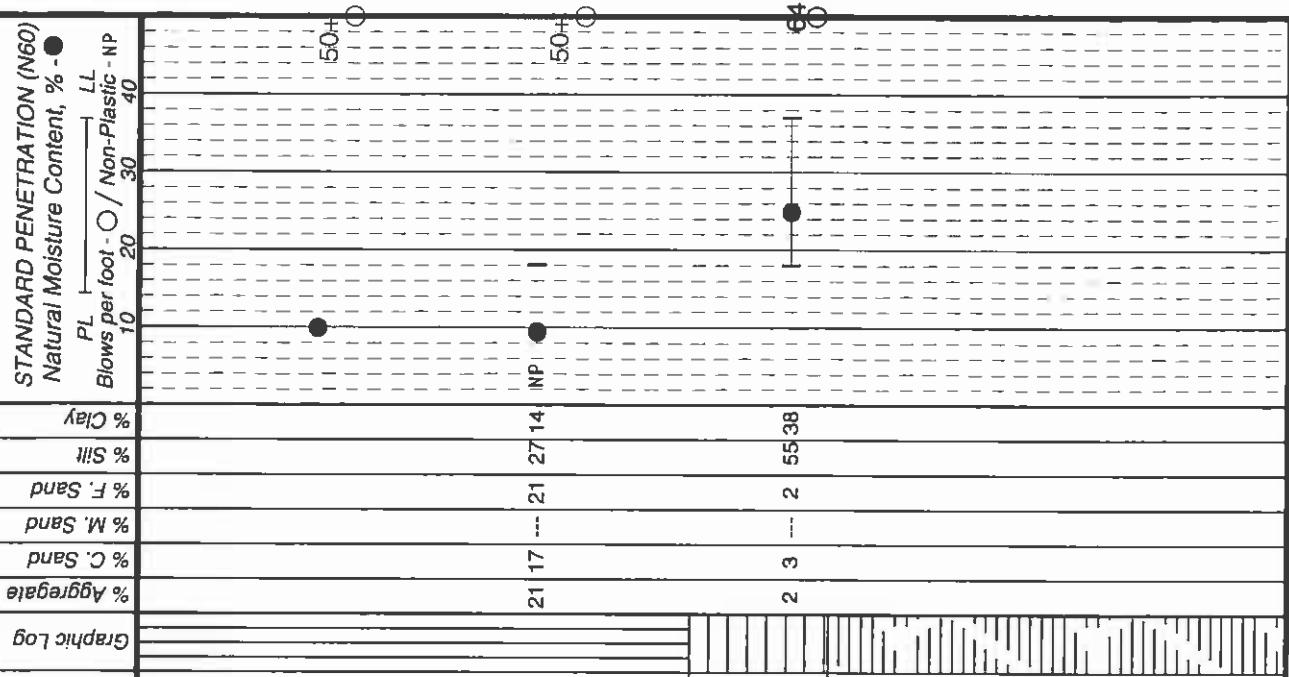
Very dense brownish gray SANDY SILT (A-4a), trace gravel; damp.

Client: ms consultants

Project: FRA-70-B-93

Job No. 0221-1004.01

LOG OF: Boring B-041		Location: Sta. 762+58.90, 115.94 ft Lt. of I-70 CL		Date Drilled: 6/23/2008 to 6/25/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Peretro-meter (tsf)	WATER OBSERVATIONS:	
		Press / Core Drive		Water seepage at: 13.0'-15.5'; 52.0'-92.0' Water level at completion: 37.1' (includes drilling water)	
		Recovery		FIELD NOTES: Advanced boring using 4.0" diameter flush joint casing.	
105	659.7	40 50/5	5 28	DESCRIPTION	
110		50/5	5 29	Very dense brownish gray SANDY SILT (A-4a), some gravel; damp.	
112.0	647.7	17	3 30	Stiff to very stiff brown and gray SILTY CLAY (A-6b), trace fine to coarse sand, trace gravel; moist.	
115.0	644.7	27 34	3 30	Shale, blue-gray, weak, highly weathered, thinly laminated, slightly calcareous, jointed, moderately to highly fractured, tight, with typical narrow fractures, slightly rough; RQD 42%, Loss 0%. Contains dark gray, well cemented shale zones [thin, dark bands]. Dark shale zones are typically 1-2 inches thick, spaced at intervals of 12 to 18 inches.	
120		Core 60"	Rec 60"	RQD 80%	R1
125	634.7	Core 60"	Rec 60"	RQD 25%	R2



Client: ms consultants

Project: FRA-70-8.93

LOG OF: Boring B-041

Job No.0221-1004.01

LOG OF: Boring B-041		Location: Sta. 762+58.90, 115.94 ft Lt. of I-70 CL		Date Drilled: 6/23/2008 to 6/25/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro-meter (tsf)	Press / Core Drive	Recovery
127.5	632.2				
130	634.7	Blows per 6"	Blows per 6"	Blows per 6"	Blows per 6"
135.0	624.7	Core 120"	Rec 120"	RQD 58%	R3

WATER OBSERVATIONS:
Water seepage at: 13.0'-15.5'; 52.0'-92.0'
Water level at completion: 37.1' (includes drilling water)

FIELD NOTES:
Advanced boring using 4.0" diameter flush joint casing.

DESCRIPTION

Shale, blue-gray, weak, highly weathered, thinly laminated, slightly calcareous, jointed, moderately to highly fractured, tight, with typical narrow fractures, slightly rough; RQD 42%, Loss 0%. Contains dark gray, well cemented shale zones [thin, dark bands]. Dark shale zones are typically 1-2 inches thick, spaced at intervals of 12 to 18 inches.

Interbedded Shale [93%] and Limestone [7%], RQD 78%, Loss 0%; Shale, dark gray, weak, highly weathered, thinly laminated, calcareous, jointed, highly fractured with typical, tight, slightly rough shear fractures; Limestone, light gray, strong, slightly weathered, pyritic, unfractured.

@ 133.5' - 134.0', qu = 1695 psi

Bottom of Boring - 135.0'

GRADATION

% Aggregate

% C. Sand

% M. Sand

% F. Sand

% Silt

% Clay

Blows per foot - O / Non-Plastic - NP

LL

PL

Natural Moisture Content, % - ●

STANDARD PENETRATION (N60)

Client: ms consultants

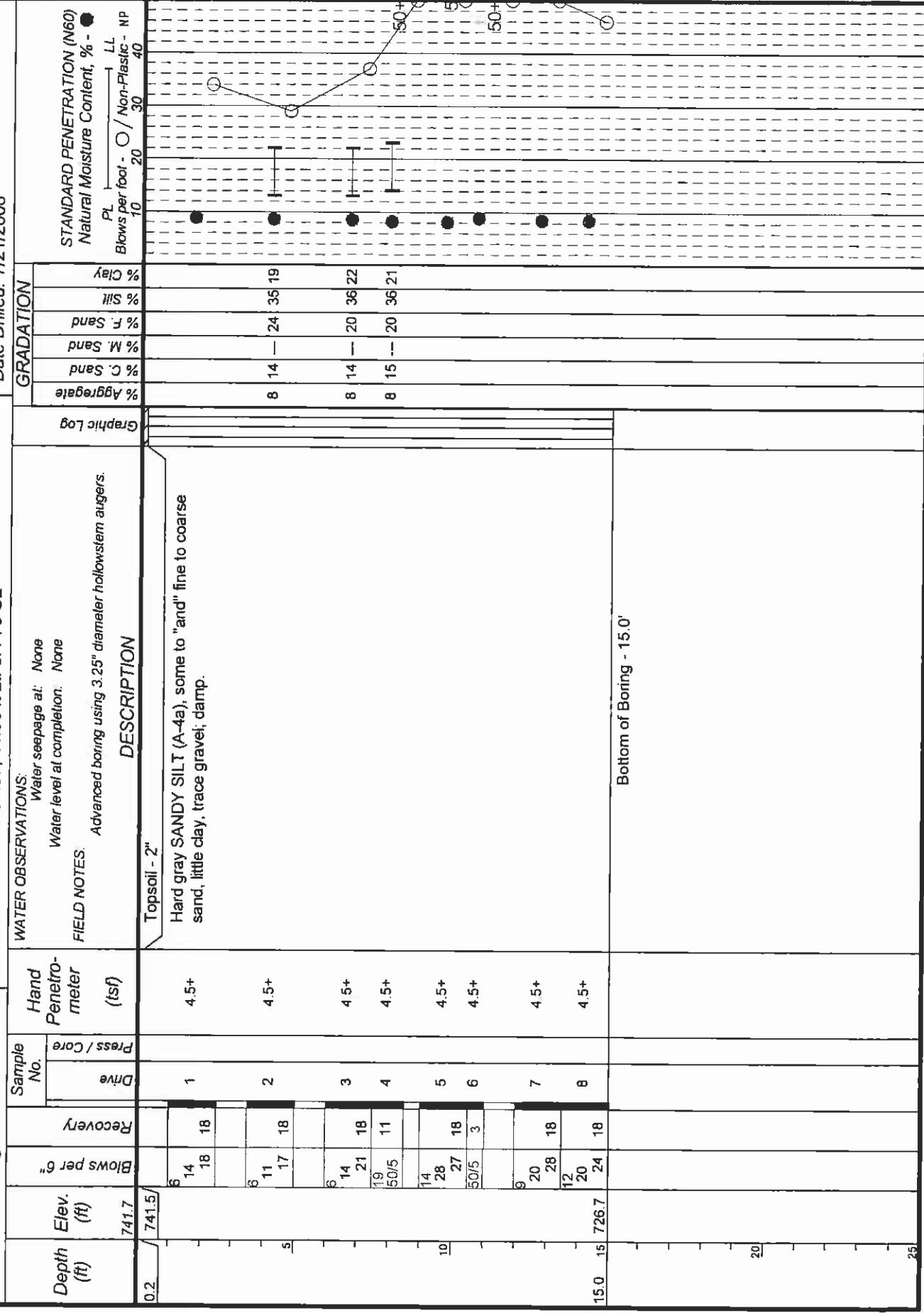
Project: FRA-70-8.93

ms

LOG OF: Boring B-042

Location: Sta. 764+57.07, 41.06 ft Lt. of I-70 CL

Date Drilled: 7/21/2008



Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-043

Location: Sta. 765+56.96, 96.30 ft Rt. of I-70 CL

WATER OBSERVATIONS:
Water seepage at: 37.0', 77.0'
Water level at completion: 14.4' (includes drilling water)

FIELD NOTES:

Advanced boring using 3.25" diameter hollowstem augers.

STANDARD PENETRATION (N60)
Natural Moisture Content, % - ●
PL ↓ 10 20 30 40
Blows per foot - O / Non-Plastic - NP
LL ↓ 10 20 30 40

DESCRIPTION

Depth (ft)	Elev. (ft)	Recovery	Sample No.	Hand Penetro- meter (fsf)	Press / Core Drive	WATER OBSERVATIONS: Water seepage at: 37.0', 77.0' Water level at completion: 14.4' (includes drilling water)	GRADATION		Date Drilled: 7/16/2008 to 7/20/2008		
							% Aggregate	% Clay	% Silt	% Sand	% M. Sand
1.9	741.2	3	8	1		Asphalt Concrete - 9" Portland Cement Concrete - 9" Aggregate Base - 5"					
3.5	739.6		13	13		Medium dense brown GRAVEL WITH SAND, SILT, AND CLAY (A-2-6), some silty clay; damp.					
5			7	7		Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.					
6.0	737.1		15	10							
			7	25		Very dense grayish brown SANDY SILT (A-4a), some fine to coarse sand, trace gravel; moist.					
			7	30							
			12	45	1	@ 7.5'-9.0', rock blocking sampler.					
9.0	734.1		10	31	5	Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.					
10.5	732.6		46	18	6	Hard gray SILT AND CLAY (A-6a), some to "and" fine to coarse sand, trace to little gravel; damp.					
			12	40	6						
			46	18	6						
			7	12	7						
			24	18	7						
			12	28	8						
15			49	10	4.5+						
			7	19	9						
			39	18	4.5+						
			39	49	10						
20			42	18	4.5+						
			12	24	11						
			42	18	4.5+						
25	718.1		17	42	12						
			21	42	18						
			17	12	18						
			12	33	20						

Graphic Log

SAND

CLAY

SILT

AGGREGATE

WATER

Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 765+56.96, 96.30 ft Rt. of I-70 CL		Date Drilled: 7/16/2008 to 7/20/2008		Job No. 02221-1004.01									
LOG OF: Boring B-043																	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (fs)	WATER OBSERVATIONS:		GRADATION		% Clay	% Silt	% Sand	% M. Sand	% C. Sand	% Aggregate	Graphic Log			
		Drive Press / Core	Blows per 6"	Water seepage at: 37.0'; 77.0' Water level at completion: 14.4' (includes drilling water)		PL Blows per foot - O / Non-Plastic - NP				LL 30 40		50+ 10					
718.1	718.1	Recovery		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.		DESCRIPTION											
				Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace gravel; damp.													

Client: ms consultants

Project: FRA-70-8-93

Job No. 0221-1004.01

LOG OFF: Boring B-043

Location: Sta. 765+56.96, 96.30 ft Rt. of I-70 CL

Date Drilled: 7/16/2008 to 7/20/2008

Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:		FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION			STANDARD PENETRATION (N60) Natural Moisture Content, % - ●
				Press / Core	Drive			% Clay	% Silt	% Sand	
55	693.1										
57.0	686.1										
		12	12	18	19						
		12	12	21	18						
		35	43	50/5	8	20	4.5+				
		50/1	1			21	--				
		14	33	36	4	22	4.5+				
		17	23	28	18	23	4.5				
75	668.1										

Blows per 6"

Recovery

Drill

Press / Core

Sample No.

Hand Penetro-meter (tsf)

Water seepage at: 37.0', 77.0'
Water level at completion: 14.4' (includes drilling water)

FIELD NOTES:

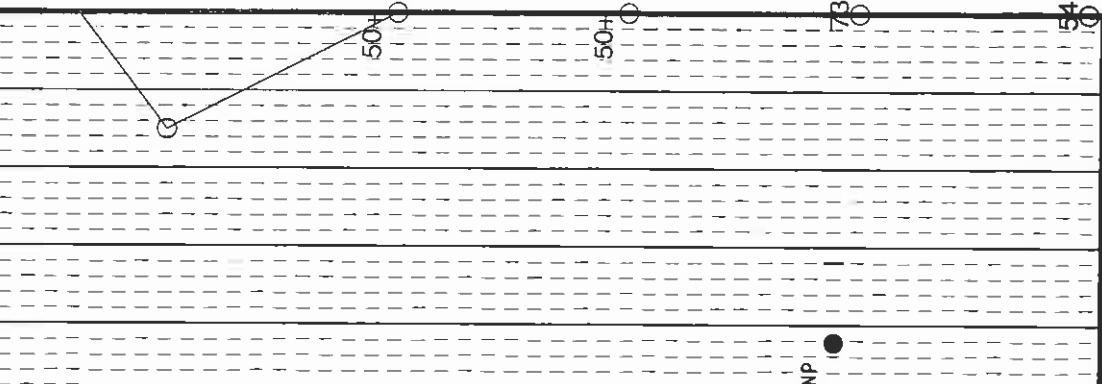
Sta. 765+56.96, 96.30 ft Rt. of I-70 CL

DESCRIPTION

Dense gray GRAVEL (A-1-a), some fine to coarse sand, trace silt; wet. (GEOLOGIST'S DESCRIPTION - sample lost)

@ 50.0'-60.0', difficult drilling, possible boulders.

@ 55.0'-60.0', encountered gas pockets.



Client: ms consultants

LOG OF: Boring B-043

Project: FRA-70-8.93

Location: Sta. 765+56.96, 96.30 ft Rt. of I-70 CL

Job No. 0221-1004.01

LOG OF: Boring B-043		WATER OBSERVATIONS:		Date Drilled: 7/16/2008 to 7/20/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro-meter (tsf)	Press / Core Drive	GRADATION
80	668.1	13	2B 50/5	17	50+ 80 60 40 20 10 0
82.0	661.1	24			50+ 80 60 40 20 10 0
85		@ 78.5', 2.0 feet sand heave.			50+ 80 60 40 20 10 0
88					50+ 80 60 40 20 10 0
90					50+ 80 60 40 20 10 0
93.5	649.6	25			50+ 80 60 40 20 10 0
95.0	648.1	26			50+ 80 60 40 20 10 0
100	643.1	27			50+ 80 60 40 20 10 0

FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.
 Water seepage at: 37.0', 77.0'
 Water level at completion: 14.4' (includes drilling water)

DESCRIPTION

Very dense gray FINE SAND (A-3), trace gravel; wet.

Very dense gray GRAVEL WITH SAND (A-1-b), some fine to coarse sand, little silt; wet.

Severely weathered gray SHALE.

Shale, blue-gray, weak, highly weathered, thinly laminated, slightly calcareous, pyritic, jointed, moderately fractured, tight, slightly rough; RQD 88%, Loss 0%. Contains dark gray, well cemented shale zones [thin, dark bands]. Dark shale zones are typically 1-2 inches thick, spaced at intervals of 12 to 18 inches.

Client: ms consultants

Project: FRA-70-8.93

Job No. 0221-1004.01

LOG OF: Boring B-044		Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL		Date Drilled: 6/16/2008 to 6/19/2008	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro-meter (tsf)	WATER OBSERVATIONS:	
		Press / Core Drive Recovery		Water seepage at: 13.5'-14.2'; 23.5'-25.0' Water level at completion: 19.1' (beginning of shift, 9/17/08) FIELD NOTES: Advanced boring using 4.25" diameter hollowstem augers to 47.0'; 4.0" casing from 47.0' to 115.5'. DESCRIPTION: Asphalt Concrete Pavement - 2" Portland Cement Concrete - 4" Aggregate Base - 10"	
1.3	- 763.3	10	--	POSSIBLE FILL: Stiff to very stiff brown SILTY CLAY (A-6b), little to some fine to coarse sand, trace gravel; moist.	
3.5	- 761.1	9	8	POSSIBLE FILL: Very stiff mottled olive-brown and gray CLAY (A-7-6), little fine to coarse sand, trace gravel; moist.	
5.5	- 759.1	5	8	Very stiff brown SANDY SILT (A-4a), trace gravel, some clay; damp.	
		4	3		
		5	11	2.5	
		5	18		
		8	4	4.0	
		12	18		
		15	16	5	2.0
		16	18		@ 11.0'-16.0', encountered very difficult drilling, dense till, possible cobbles. Recovered rock fragments in split spoon.
10.0	- 753.6	12	13		Dense brown GRAVEL WITH SAND AND SILT (A-2-4); damp.
		21	17	6	
13.0	- 751.6	13	21		Stiff gray SILT AND CLAY (A-6a), some fine to coarse sand, little to some gravel; damp to moist.
		13	21		Very dense brown GRAVEL (A-1-a), little silty clay; moist to wet. @ 14.2', refusal of split spoon.
14.0	- 750.6	18	8	7A	
		50/2		7B	
15.0	- 749.6				
		20	16	8	4.0
18.0	- 746.6				Very stiff to hard gray SANDY SILT (A-4a), little to some clay, trace to little gravel; damp.
		9	14	9	3.5
		14	18	10	Very stiff to hard gray SILT AND CLAY (A-6a), little to some clay, little gravel; damp to moist.
		17	19	11	
25	- 739.6	28	35	18	@ 20.5', moist to wet.
		35	18		

Client: ms consultants		Project: FRA-70-8.93		LOG OF: Boring B-044		Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL		Date Drilled: 6/16/2008 to 6/19/2008		Job No. 00221-1004.01	
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetrometer (fsf)	WATER OBSERVATIONS:	FIELD NOTES:	Gradation	GRADATION	STANDARD PENETRATION (N60)	Natural Moisture Content, % -	PL	LL
		Drive	Press / Core Recovery	Water seepage at: 13.5'-14.2'; 23.5'-25.0' Water level at completion: 19.1' (beginning of shift, 9/17/08) 20.8' (includes drilling water)	Advanced boring using 4.25" diameter hollowstem augers to 47.0'; 4.0" casing from 47.0' to 115.5'. DESCRIPTION	% Clay	% Silt	Blows per foot - O / Non-Plastic - NP	30	40	50+
28.0	736.6	12 20 24 28	12 17 18	3.0	Very stiff to hard gray SILT AND CLAY (A-6a), little to some clay, little gravel; damp. @ 26.0', contains few thin (less than 1") fine to medium grained sand seams.	18	12	20	27	23	
30		13 15 25	13 18	4.0	Very stiff to hard gray SANDY SILT (A-4a), little to some clay, little gravel; damp.	18	12	20	27	23	
35		34 32	14 4	--	@ 33.9', encountered possible cobble or boulder.	18	12	20	27	23	
39.6	725.0	14 37 48 18 15A 15B	14 48 18 15A 15B	3.5	@ 36.0', pulled augers to change lead. Lead auger bit destroyed. Could not advance further with 500-600 psi down pressure. Changed tools to drill with casing.	18	12	20	27	23	
42.0	722.6				Very dense gray COARSE AND FINE SAND (A-3a), little silty clay, trace gravel; wet.	18	12	20	27	23	
45		21 42 50/5	16 17	4.5+	Hard gray SANDY SILT (A-4a), "and" fine to coarse sand, trace gravel; damp.	18	13	21	35	23	NP
47.0	717.6				Very dense gray GRAVEL WITH SAND (A-1-b), some silty clay; wet.	18	13	21	35	23	50+
50	714.6	10 20 42	6	4.6		18	13	21	35	23	-23--

Client: ms consultants		Project: FRA-70-8.93		Date Drilled: 6/16/2008 to 6/19/2008	
LOG OF: Boring B-044		Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL			
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	WATER OBSERVATIONS:	
		Drive Recovery	Press / Core (tsf)	Water seepage at: 13.5'-14.2', 23.5'-25.0' Water level at completion: 19.1' (beginning of shift, 9/17/08) 20.8' (includes drilling water)	
52.0	712.6			FIELD NOTES: Advanced boring using 4.25" diameter hollowstem augers to 47.0'; 4.0" casing from 47.0' to 115.5'. DESCRIPTION	
				Very dense gray GRAVEL WITH SAND (A-1-b), some silty clay; wet.	
				Hard gray SANDY SILT (A-4a), some fine to coarse sand, trace to little gravel; damp.	
				@ 62.0 to 67.0', very stiff.	
55		14 42 43 6	18	4.5+	
55		20 35 38 18	19	4.5+	
60					2.5
65		14 22 31 18	20		
70		15 26 42 18	21	4.5+	
75		22 26 34 18	22	4.5+	

Client: ms consultants

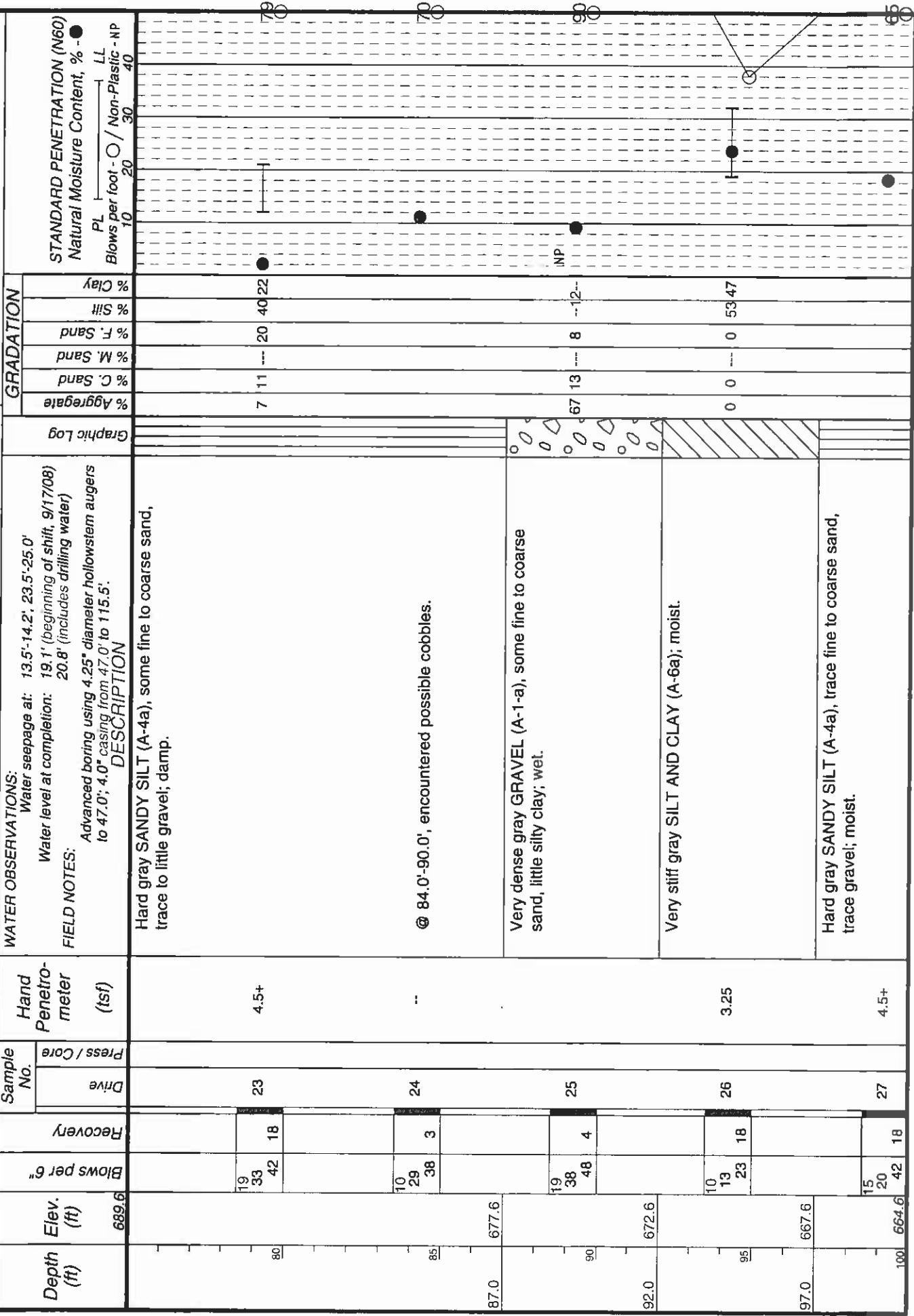
Project: FRA-70-8.93

LOG OF: Boring B-044

Job No. 0221-1004.01

Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL

Date Drilled: 6/16/2008 to 6/19/2008



Client: ms consultants		Project: FRA-70-8.93		Location: Sta. 767+54.82, 120.59 ft Lt. of I-70 CL		Date Drilled: 6/16/2008 to 6/19/2008		Job No. 0221-1004.01	
LOG OF: Boring B-044									
Depth (ft)	Elev. (ft)	Sample No.	Hand Penetro- meter (tsf)	Press / Core	Drive	Recovery	GRADATION	STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL - □ LL - ▲ Non-Plastic - NP Blows per foot - O	
105	664.6	13 14 14	28 18	2.5					
107.0	667.6								
110.0	664.6	42 50/3	9 29	4.5+					
111.5	653.1	20 37 50/3	30						
114.9	649.7	Core 60"	Rec 60"	RQD 73%	R1				
120	639.6	Core 60"	Rec 60"	RQD 77%	R2				

WATER OBSERVATIONS: Water seepage at: 13.5'-14.2', 23.5'-25.0'
Water level at completion: 19.1' (beginning of shift, 9/17/08)
20.8' (includes drilling water)

FIELD NOTES: Advanced boring using 4.25" diameter hollowstem augers
to 47.0'; 4.0" casing from 47.0' to 115.5'.
DESCRIPTION

Hard gray SILT (A-4b), trace fine to coarse sand, trace gravel; moist.
@ 102', very stiff, grayish brown.

Hard brown and gray CLAY (A-7-6), varved; damp.

Shale, grayish brown, very weak, severely weathered, thinly laminated.

Shale [90%] interbedded and Siltstone [10%], RQD 76%, Loss 0%; Shale, blue-gray, very weak to weak, severely weathered, laminated, slightly calcareous, jointed, moderately fractured, tight, slightly rough, typical clay-filled fractures; Siltstone, gray to dark gray, strong, slightly weathered, laminated, argillaceous.
@ 112.0', high angle fracture, clay filled with fine and coarse grained sand.

Shale, blue-gray, weak, moderately to highly weathered, laminated, slightly calcareous, jointed, slightly fractured, tight, slightly rough, clay filled fractures; RQD 77%, Loss 0%. Contains dark gray, well cemented shale zones [thin, dark bands]. Dark shale zones are typically 1-2 inches thick, spaced at intervals of 12 to 18 inches.
@ 114.4', 115.3', 116.7', 118.4', and 119.4', high angle fractures.
@ 117.6' - 118.0', qu = 1202 psi

Client: ms consultants

Project: FRA-70-8-93

ms

Job No. 0221-1004.01

LOG OF: Boring B-045		Location: Sta. 768+75.91, 91.18 ft Rt. of I-70 CL		Date Drilled: 7/17/2008							
Depth (ft)	Elev. (ft)	Sample No.	Hand Petro- meter (sf)	WATER OBSERVATIONS Water seepage at: Water level at completion:	FIELD NOTES: Advanced boring using 3.25" diameter hollowstem augers.	DESCRIPTION	GRADATION		STANDARD PENETRATION (N60) Natural Moisture Content, % - ● PL - □ / Non-Plastic - NP Blows per foot - ○ / Non-Plastic - NP 20 30 40 10		
		Recovery Blows per 6"	Press / Core				% Aggregate	% C Sand	% M. Sand	% F. Sand	% Silt
1.1	744.7	7 9 9	1 14	4.5+		Asphalt Concrete - 8" Aggregate Base - 5"					
						Hard gray SANDY SILT (A-4a), little clay, trace to little gravel, damp.					
5	745.8	7 9 9	2 10	4.5+							
8.0	737.8	7 12 15	3 11 12	4.0	@ 5.6', sand and gravel seam.		12 14 - 22 32 20	9 24 - 21 30 16			
10.0	730.8	13 16 16	5 17 18	4.5+	Hard gray SILT AND CLAY (A-6a), some lo "and" fine to coarse sand, trace gravel; damp. @ 8.0-9.5', contains sand seams.		7 15 -- 20 37 21				
15.0	730.8	16 27 24	7 27 23	4.5+							
						Bottom of Boring - 15.0'					