# STRUCTURE ESTIMATED QUANTITIES

Bridge No. FRA-70-1395C S. Front Street over I-70/71

> FRA-70/71-12.68/14.86 PID No. 105523

Franklin County, Ohio

## **Prepared For:**

The Ohio Department of Transportation
District 6



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Project:Bridge No. FRA-70-1395CDesign:RFVSubject:Estimated Quantities - FINALCheck:DJC

Date: 5/28/2019

## ITEM 202 - STRUCTURE REMOVED, OVER 20 FOOT SPAN

area = 11616.79 sf unit cost = \$18.00 per sf

Lump sum =  $\frac{$209,102}{}$ 

## **ITEM 202 - APPROACH SLAB REMOVED**

length = 25 ft width = 60 ft

> Total = <u>334</u> sy (+ Sidwalk Curves) <u>336</u> sy

## **ITEM 202 - WEARING COURSE REMOVED**

length = 190.52 ft width = 60 ft

Total = <u>1271</u> sy

## ITEM 503 - COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN

### **ITEM 503 - UNCLASSIFIED EXCAVATION**

Rear Abutment:

length = 156.94 ft Note: width = 27 ft Excav

width = 27 ft Excavation for abutments will be taken up when the exist. abutments are removed

Forward Abutment:

length = 151.77 ft width = 22 ft depth = 28.5 ft

## Abutment Subtotal = 6781 cy

Piers:

length = 148 ft
width = 17 ft
depth = 11 ft
no. of ftgs per pier = 1
no. of piers = 1

Pier Subtotal = 1025 cy

Total = 7806 cy

unit cost = \$ 40.00 per cy

Lump sum = \$312,300

## **ITEM 509 - EPOXY COATED REINFORCING STEEL**

Slab (parapets) = 17,185 lbs Slab (bridge) = 182,944 lbs Slab (east cap) = 142,393 lbs Sidewalk = 13,199 lbs Superstructure subtotal = 355,721 lbs Rear Abutment = 397,665 lbs Frwd. Abutment = 602,274 lbs 999,939 lbs Abutment subtotal = Pier & Footing = 104,135 lbs 104,135 lbs Pier subtotal = Approach slabs = 112,635 lbs Approach slab subtotal = 112,635 lbs

Total =  $\frac{1,572,430}{1}$  lbs

## ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK

## **BRIDGE**

Deck:		
thickness =	9.25	in
edge of deck		
to bridge limits =	1.25	ft
total sum of spans =	196.25	ft
total length =	198.82	
O/O of deck width =	103.25	ft
Deck Volume =	586	су
Haunch:		
t/flange width =	18	in
t/deck to t/web =	13	in

t/deck to t/web = 13 in
haunch thick. = 2.5 in
t/flange thick. (avg) = 1.25 in
total no. of beams = 10

Haunch Volume (Interior Beams) =

Haunch Volume (Interior Beams) = 32 cy Haunch Volume (Exterior Beams) = 6 cy

## Cantilever:

cantilever length (right) = 2.88 ft cantilever length (left) = 2.88 ft Cantilever Volume = 10 cy

## Signal Pole Diaphragms (Bridge):

length = 10.83 ft
thickness = 3.00 ft
height = 2.80 ft
number = 3.00 each
Diaph. Volume = 10 cy

BRIDGE TOTAL = 644 cy

### **CAP**

## Deck:

thickness = 8 in edge of deck to bridge limits = 1.25 ft total sum of spans = 196.25 ft

total length =	198.75
O/O of deck width =	40 ft
Deck volume =	196 cy

#### Haunch:

Middle Section

t/flange width = 18 in t/deck to t/web = 20.25 in haunch thick. = 11.08 in t/flange thick. (avg) = 1.17 in total no. of beams = 5 haunch length = 141.25 ft

Haunch Volume (Interior Beams) = 34 cy Haunch Volume (Exterior Beams) = 18 cy

**End Sections** 

t/flange width = 18 in t/deck to t/web = 12 in haunch thick. = 2.83 in t/flange thick. (avg) = 1.17 in total no. of beams = haunch length = 55 ft

Haunch Volume (Interior Beams) = 4 cy Haunch Volume (Exterior Beams) = 2 cy

Haunch Volume = 58 cy

### Cantilever:

cantilever length (right) = 2.00 ft
cantilever length (left) = 2.00 ft

Middle Section 13 cy
End Sections 2 cy
Cantilever Volume = 15 cy

Slab at Ends of Span

area = 2302.59 sf avg. thickness = 8.50 in

Cap End Slab Volume = 60 cy

#### Signal Pole Diaphragms (East Cap):

length = 9.00 ft thickness = 3.00 ft height = 2.80 ft number = 1.00 each Diaph. Volume = 3 cy

CAP TOTAL = 333 cy

Total = <u>977</u> cy

## ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET), AS PER PLAN

area (west) = 4.42 sf area (east) = 4.42 sf length (west) = 132.04 ft length (east) = 144.89 ft pylons = 924.00 cf

Total = 80 cy

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS

wall area = 3238.52 sf thickness = 3.00 ft window area = 436.90 sf window thickness = 2.00 ft pedestals = 32.32 cf

Total = 393 cy

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING, AS PER PLAN

Skew angle = 12.97 degrees

#### **Rear Abutment:**

Beam Seat

beam seat area (above paneling)
beam seat width =

lower beam seat area =

lower beam seat width =

Beam Seat Volume =

634.99 sf (cad)
634.99 sf (cad)
6575 ft
156.94 sf (cad)
5.50 ft
5149.34 cf

Backwall

backwall area = 682.99 sf (cad)
backwall thickness = 1.75 ft
approach slab thickness = 1.25 ft
abutment length = 156.94 ft
Back Wall Volume = 1097.14 cf

Additonal Volume

pedestals = 16.92 cf end wall volumes = 191.79 cf Rear Abutment Subtotal 239 cy

#### Forward Abutment:

Beam Seat

beam seat area (above paneling)
beam seat width =

lower beam seat area =

lower beam seat width =

Beam Seat Volume =

527.19 sf (cad)

6.75 ft

139.76

5.50 ft

4327.21 cf

Backwall

backwall area = 673.50 sf (cad)
backwall thickness = 1.75 ft
approach slab thickness = 1.25 ft
abutment length = 151.77 ft
Back Wall Volume = 1083.77 cf

Additonal Volume

pedestals = 16.92 cf end wall volumes = 105.26 cf Forward Abutment Subtotal = 205 cy

Total = 444 cy

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, FOOTING, AS PER PLAN

Pier:

height = 3 ft width = 15 ft length = 147.66 ft

Total =  $\frac{247}{}$  cy

### ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, SIDEWALK, AS PER PLAN

### Sidewalk on BRIDGE:

area = 6042.46 sf avg. thickness = 8.25 in Sidewalk Volume = 154 cy

### Sidewalk on CAP (sidewalk portion over end sections only):

area = 1526.61 sf avg. thickness = 8.25 in Sidewalk Volume = 39 cy

Total = <u>193</u> cy

## ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)

 Rear Abutment:
 Face Top
 2967.00 sf 863.17 sf 863.17 sf 4546.72 sf 1012.00 sf 2649.34 sf

 Forward Abutment :
 Face Top
 1012.00 sf 2649.34 sf

Total =  $\frac{1,338}{1}$  sy

### **ITEM 512 - TYPE 2 WATERPROOFING**

Rear Abutment = 9.68 ft Forward Abutment = 8.47 ft

Total =  $\frac{7}{2}$  sy

## ITEM 512 - SEALING OF CONCRETE SURFACES (NON-EPOXY)

west parapet = 906.90 sf east parapet = 1155.07 sf pylons = 451.87 sf

west sidewalk = 2906.80 sf (includes small area of sidewalk on SW abutment)

east sidewalk = 2388.40 sf

east cap = 3539.28 sf ====> cap total = 394 sy

Total = <u>1261</u> sy

## ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 4

BRIDGE				
Girders:				
Section 1				
length =	65.50	ft		
weight =	225.43	lbs/ft		
Section 2				
volume =	43.53	cf		
unit weight steel =	21327.4	lbs		
Section 3				
length =	67.25	ft		
weight =	225.43	lbs/ft		
Detail factor =	1.0			
No. of beams =	10			
D 0 11111	540.000			
Beam Subtotal =	512,600	IDS		
Calingo	# of plates	longth (in)	width (in)	thick (in)
<b>Splices:</b> Top Flange	# or plates	length (in)	width (in)	thick (in)
outside plates =	1	37	18	0.625
outside plates –		01	10	0.023

inside plates =	2	37	8	0.625
Bottom Flange				
outside plates =	1	44	18	0.75
inside plates =	2	44	8	0.75
Web				
plates =	2	29	22.5	0.5

Plate weight/splice = 726 lbs

Splice Bolts	# of bolts	length (in)	bolt wt.	washer wt.
Top Flange =	40	1.5	148	11.3
Bot. Flange =	48	1.5	148	11.3
Web =	48	1.5	148	11.3

<sup>\*</sup> from steel manual

Bolt + Washer weight

 Top Flange =
 64 lbs

 Bot. Flange =
 76 lbs

 Web =
 76 lbs

Total weight/splice = 943 lbs No. of splices = 20

## Splice + Bolts Subtotal = 18,856 lbs

#### Intermediate Crossframes:

Length of Diagonals = 10.67 ft (weighted avg.)

No. of Diagonals = 2 Length of Horiz. = 2 10.62 ft (weighted avg.)

No. of Horiz. = 1 Angle weight / ft. = 9.80 lbs/ft

Crossframe weight = 313 lbs ==> per x-frame assembly

x-frame stiffeners? y y or n

Length = 34.000 in ==> web depth
Width = 5.000 in
Thickness = 0.375 in

Stiffener weight = 36 lbs ==> per x-frame assembly

Total Intermediate Crossframe

Assembly Weight = 349 lbs. ==> per x-frame assembly

No. of assemblies = 145

#### **End Crossframes:**

Length of Diagonals = 10.85 ft (weighted avg.) No. of Diagonals = 2

Length of Horiz. = 10.78 ft (weighted avg.)

No. of Horiz. = 1 Angle weight / ft. = 9.80 lbs/ft

Crossframe weight = 318 lbs. ==> per x-frame assembly

x-frame stiffeners? y y or n

Length = 5.000 in ==> web depth

Width = 34.000 in Thickness = 0.375 in

Stiffener weight = 36 lbs ==> per x-frame assembly

<sup>\*</sup> washer weight is per 100 ct.

Total x-frame End Assembly Weight = 354 lbs ==> per x-frame assembly No. of assemblies = Crossframe Subtotal = 57,038 lbs Signal Support Diaphragm: Embedded steel plate (20"x20"x1.75") = Support angles (L6x4x1/2) - Length = Support angle weight per ft = 198 lbs 10.833 ft 16.2 lb/ft No. of support angles = 2 ea No. of signal support diaphragms = 3 ea Signal support subtotal = 1,648 lbs **BRIDGE STRUCTURAL STEEL =** 590,142 lbs

CAP				
Girders:				
Section 1				
length =	65.50	ft		
weight =	202.47			
Section 2	202.47	105/11		
volume =	42.49	of		
unit weight steel =	20820	IDS		
Section 3	C7.0F	tı.		
length =	67.25			
weight =	202.47	IDS/IT		
Detail factor =	1.0			
Detail factor =	1.0			
No. of beams =	5			
No. of bearis =	3			
Beam subtotal =	238,500	lhe		
Beam Subtotal =	200,000	103		
Splices:	# of plates	length (in)	width (in)	thick (in)
Top Flange	" or platee	iongui (iii)	wider (iii)	timore (m)
outside plates =	1	37	18	0.625
inside plates =	2	37	8	0.625
Bottom Flange	_	O1	Ū	0.020
outside plates =	1	44	18	0.75
inside plates =	2	44	8	0.75
Web	_		Ū	0.70
plates =	2	29	22.5	0.5
piates =	_	20	LL.U	0.5
Plate weight/splice =	726	lhs		
r iato woight opiloo =	, 20			
Splices Bolts	# of bolts	length (in)	bolt wt.	washer wt.
Top Flange =	40	1.5	148	11.3
Bot. Flange =	48	1.5	148	11.3
Web =	48	1.5	148	11.3
	* from stee			
		eight is per 1	00 ct.	
		olgili lo poi l	00 01.	
Bolt + Washer weight				
Top Flange =		lbs		
Bot. Flange =		lbs		
Web =	_	lbs		
	.0			
Total weight/splice =	943	lbs		
No. of splices =	10			
140. 01 opilo00 =	10			

Splice subtotal =	9,428 lbs ==> plates + bolts
ntermediate Crossfr	ames:
Length of Diagonals =	8.91 ft (weighted avg.)
No. of Diagonals =	2
Length of Horiz. =	8.82 ft (weighted avg.)
No. of Horiz. =	1
Angle weight / ft. =	9.80 lbs/ft
Crossframe weight =	261 lbs ==> per x-frame assembly
x-frame stiffeners?	y y or n
Length =	34.000 in ==> web depth
Width =	5.000 in
Thickness =	0.375 in
Stiffener weight =	36 lbs ==> per x-frame assembly
Total x-frame	
assembly weight =	297 lbs ==> per x-frame assembly
No. of assemblies =	65
End Crossframes:	
Length of Diagonals =	9.00 ft (weighted avg.)
No. of Diagonals =	2
Length of Horiz. =	8.90 ft (weighted avg.)
No. of Horiz. =	1
Angle weight / ft. =	9.80 lbs/ft
Crossframe weight =	264 lbs ==> per x-frame assembly
x-frame stiffeners?	y y or n
Length =	34.000 in ==> web depth
Width =	5.000 in
Thickness =	0.375 in
Stiffener weight =	36 lbs ==> per x-frame assembly
Total x-frame	
assembly weight =	300 lbs ==> per x-frame assembly
No. of assemblies =	8
Crossframe Subtotal	l = 21,715 lbs
	,
Signal Support Diapl	
Embedded steel plate	
Support angles (L6x4x	x1/2) - Length = 9.000 ft ngle weight per ft = 16.2 lb/ft
	port angles = 2 ea
No. of signal support of	
Signal sur	oport subtotal = 490 lbs
	•
CAP STRUCTURAL S	STEEL = 270,133 lbs

Total = 860,275 lbs

## ITEM 513 - WELDED STUD SHEAR CONNECTORS

Bridge No. rows per beam = 259 No. per row = 3 No. of beams = 10

> Number per signal support = 50 Number of signal supports = 3

Bridge Total = 7920 ea

Cap No. rows per beam = 277
No. per row = 3
No. of beams = 5

Number per signal support = 50 Number of signal supports = 1

Bridge Total = 4205 ea

Total = 12,125 ea

# ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, FINISH COAT

Bridge web area = 1208.53 sf section 1 flange area = 315.22 sf 334.06 sf section 2 flange area = section 3 flange area = 323.64 sf No. of beams = 10 Crossframes = 6958.45 sf Stiffeners = 827.45 sf Utility Supports = 564.04 sf Bridge Subtotal = 30,164 sf

 Cap web area =
 1208.53 sf

 section 1 flange area =
 311.13 sf

 section 2 flange area =
 332.67 sf

 section 3 flange area =
 319.44 sf

 No. of beams =
 5

 Crossframes =
 2595.30 sf

 Stiffeners =
 370.58 sf

 Cap Subtotal =
 13,825 sf

Total =  $\frac{43,989}{}$  sf

### **ITEM 514 - FINAL INSPECTION REPAIR**

Length = 198.75 ft
No. Girders = 15 ea
No. Crossframes = 236 ea

Total = 32 ea

## ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL (3")

Length = 294.00 ft

Total = 294 ft

## ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL (5")

Length = 198.82 ft

Total = <u>199</u> ft

#### ITEM 516 - 1" PREFORMED EXPANSION JOINT FILLER

Rear	Αb	utm	en	t:
------	----	-----	----	----

beam seat height = 5.33 ft
beam seat width = 6.75 ft
beam seat length = 156.94 ft
wall height = 17.65 ft
wall width = 0.83 ft
backwall height = 4.31 ft
backwall width = 1.75 ft

### **Forward Abutment:**

beam seat height = beam seat width = 6.75 ft
beam seat length = 151.77 ft
wall height = 20.55 ft
wall width = 0.83 ft
backwall height = 4.44 ft
backwall width = 1.75 ft

Total = 398 sf

#### ITEM 516 - 2" PREFORMED EXPANSION JOINT FILLER

Pier:

height = 24.24 ft

Total =  $\frac{73}{5}$  sf

# ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES & LOAD PLATE (NEOPRENE) 10.5" x 1'-4" x 2.45" PAD WITH 11.5" x 1'-10" BEVELED PLATE, AS PER PLAN

Rear Abutment = 15 ea Forward Abutment = 15 ea

Total = 30 ea

# ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES & LOAD PLATE (NEOPRENE) 1'-6" x 2'-0" x 3.40" PAD WITH 1'-7" x 2'-9" BEVELED PLATE, AS PER PLAN

Pier = 15 ea

Total = 15 ea

#### ITEM 518 - POROUS BACKFILL WITH GEOTEXTILE FABRIC

#### **Rear Abutment:**

avg. top of backwall = 753.73 top of drilled shaft = 744.45 height = 7.53 ft length = 156.94 ft thickness = 2.00 ft area (panel footing) = 6.58 sf

Rear Abutment Subtotal = 3,397 cf

## **Forward Abutment:**

avg. top of backwall = 757.48 top of drilled shaft = 748.35 height = 7.38 ft length = 151.77 ft thickness = 2.00 ft area (panel footing) = 6.58 sf

Forward Abutment Subtotal = 3,238 cf

> Total = 246 cy

### ITEM 518 - 6" PERFORATED CORRUGATED PLASTIC PIPE

**Rear Abutment:** 

length = 313.88 ft (inlcudes façade panel pipe)

**Forward Abutment:** 

303.54 ft (inlcudes façade panel pipe) length =

> Total = 618 ft

## ITEM 518 - 6" NON-PERFORATED CORRUGATED PLASTIC PIPE

**Rear Abutment:** 

length = 20 ft

**Forward Abutment:** 

length = 27 ft

> Total = 47 ft

#### ITEM 524 - DRILLED SHAFTS, 60" DIAMETER, ABOVE BEDROCK

**Rear Abutment:** 

length = 54.45 ft no. of columns = 31

**Forward Abutment:** 

58.35 ft length = no. of columns = 45

> Total = 4,314 ft

### ITEM 526 - REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=15"), AS PER PLAN

Rear Approach Slab:

3957.64 sf area =

Forward Approach Slab:

area = 4656.49 sf

> Total = 958 sy

## ITEM SPECIAL - STRUCTURE, MISC.: PERMANENT UTILITY SUPPORTS

Length of Horiz. = 10.78 ft (weighted avg.)

No. of Horiz. =

Angle weight / ft. = 12.80 lbs/ft

utility support weight = 138 lbs ==> per assembly

Stiffeners?

y y or n 5.000 in ==> web depth Length =

Width = 34.000 in 0.375 in Thickness =

Stiffener weight = 36 lbs ==> per x-frame assembly

Total utility support

assembly weight = 174 lbs ==> per x-frame assembly

No. of assemblies =

Total Weight = Cost =

5050 lbs \$1.50 per lb

 $Total = \frac{$7,576.00}{}$ 

## ITEM SPECIAL - STRUCTURE, MISC.: PRECAST FAÇADE PANELS

**Rear Abutment:** 

area = 2769.95 sf

Forward Abutment:

area = 3261.59 sf

Total =  $\frac{6.032}{1}$  sf

## ITEM 607 - FENCE, MISC.: WALL MOUNTED TYPE A (W/ VANDAL MESH)

**Rear Abutment:** 

length = 4.5 ft ==> on SW knee wall extension

North end of west parapet:

length = 15.50 ft ==> north of NW end pilaster

Total = 20 ft

# ITEM 625 - LIGHT POLE ANCHOR BOLTS, MISC.: COMBINATION SIGNAL POLE AND PEDESTRIAN POLE ANCHOR BOLT ASSEMBLIES EMBEDDED IN CONCRETE BRIDGE DECK

Total = 8 ea