# STRUCTURE ESTIMATED QUANTITIES 

Bridge No. FRA-70-1395C<br>S. Front Street over I-70/71

FRA-70/71-12.68/14.86
PID No. 105523

Franklin County, Ohio

Prepared For:
The Ohio Department of Transportation District 6

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January 31, 2019
REVISED May 31, 2019

| Project: | Bridge No. FRA-70-1395C | Design: | RFV |
| :--- | :--- | :--- | :--- |
| Subject: | Estimated Quantities - FINAL | Check: | DJC |
| Date: | $5 / 28 / 2019$ |  |  |

## ITEM 202 - STRUCTURE REMOVED, OVER 20 FOOT SPAN

```
area = 11616.79 sf
unit cost = $18.00 per sf
```

Lump sum = \$209,102

## ITEM 202 - APPROACH SLAB REMOVED

| length $=$ | 25 ft |
| :--- | :--- |
| width $=$ | 60 ft |

Total $=$
(+ Sidwalk Curves) $\quad \underline{\underline{334} \text { sy }}$ sy

ITEM 202 - WEARING COURSE REMOVED

| length $=$ | 190.52 ft |
| :--- | ---: |
| width $=$ | 60 ft |

Total $=\quad \underline{1271} \mathbf{~ s y}$
ITEM 503 - COFFERDAMS AND EXCAVATION BRACING, AS PER PLAN

## ITEM 503 - UNCLASSIFIED EXCAVATION

| Rear Abutment: |  |  |
| :--- | ---: | :--- |
| length $=$ | 156.94 ft | Note: |
| width $=$ | 27 ft | Excavation for abutments will |
| depth $=$ | 20.75 ft | be taken up when the exist. <br> abutments are removed |

Forward Abutment:

| length $=$ | 151.77 ft |
| :--- | ---: |
| width $=$ | 22 ft |
| depth $=$ | 28.5 ft |

Abutment Subtotal $=\quad 6781$ cy


## ITEM 509 - EPOXY COATED REINFORCING STEEL

|  | Slab (parapets) = | 17,185 lbs |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Slab (bridge) = | 182,944 lbs |  |  |
|  | Slab (east cap) = | 142,393 lbs |  |  |
|  | Sidewalk = | 13,199 lbs |  |  |
| Superstructure subtotal = |  | 355,721 lbs |  |  |
|  | Rear Abutment = | 397,665 lbs |  |  |
|  | Frwd. Abutment = | 602,274 lbs |  |  |
|  | Abutment subtotal $=$ | 999,939 lbs |  |  |
|  | Pier \& Footing $=$ | 104,135 lbs |  |  |
|  | Pier subtotal $=$ | 104,135 lbs |  |  |
| Approach slabs = Approach slab subtotal $=$ |  | 112,635 lbs |  |  |
|  |  | 112,635 lbs |  |  |
|  |  | Total $=\mathbf{1 , 5 7 2 , 4 3 0} \mathrm{lbs}$ |  |  |
| ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK |  |  |  |  |
| BRIDGE |  |  |  |  |
| Deck: |  |  |  |  |
|  | thickness = | 9.25 in |  |  |
|  | edge of deck |  |  |  |
|  | to bridge limits = | 1.25 ft |  |  |
|  | total sum of spans = | 196.25 ft |  |  |
|  | total length = | 198.82 |  |  |
|  | O/O of deck width = | 103.25 ft |  |  |
|  | Deck Volume = | 586 cy |  |  |
| Haunch: |  |  |  |  |
|  | t/flange width $=$ | 18 in |  |  |
|  | $\mathrm{t} / \mathrm{deck}$ to $\mathrm{t} / \mathrm{web}=$ | 13 in |  |  |
|  | haunch thick. = | 2.5 in |  |  |
|  | t/flange thick. (avg) = | 1.25 in |  |  |
|  | total no. of beams = | 10 |  |  |
| Haunch Volume (Interior Beams) = 32 cy |  |  |  |  |
| Haunch Volume (Exterior Beams) = 6 cy |  |  |  |  |
| Cantilever: |  |  |  |  |
| cantilever length (right) = 2.88 ft |  |  |  |  |
| cantilever length (left) $=\quad 2.88 \mathrm{ft}$ |  |  |  |  |
| Cantilever Volume = 10 cy |  |  |  |  |
| Signal Pole Diaphragms (Bridge): |  |  |  |  |
|  | length = | 10.83 ft |  |  |
|  | thickness = | 3.00 ft |  |  |
|  | height $=$ | 2.80 ft |  |  |
|  | number = | 3.00 each |  |  |
|  | Diaph. Volume = | 10 cy |  |  |
|  | BRIDGE TOTAL = | 644 cy |  |  |
| CAP |  |  |  |  |
| Deck: |  |  |  |  |
|  | thickness = | 8 in |  |  |
|  | edge of deck |  |  |  |
|  | to bridge limits = | 1.25 ft |  |  |
|  | total sum of spans = | 196.25 ft |  |  |



## ITEM 511 - CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET), AS PER PLAN

| area (west) = | 4.42 sf |  |
| :---: | :---: | :---: |
| area (east) = | 4.42 sf |  |
| length (west) = | 132.04 ft |  |
| length (east) $=$ | 144.89 ft |  |
| pylons = | 924.00 cf |  |
|  | Total $=$ | $\underline{80}$ cy |

## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS

| wall area $=$ | 3238.52 sf |
| :--- | ---: |
| thickness $=$ | 3.00 ft |

```
\begin{tabular}{lr} 
window area = & 436.90 sf \\
window thickness \(=\) & 2.00 ft \\
pedestals = & 32.32 cf
\end{tabular}
Total \(=\)
```


## TEM 511 - CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING, AS PER PLAN

```
Skew angle \(=\quad 12.97\) degrees
Rear Abutment:
Beam Seat
\begin{tabular}{lc} 
beam seat area (above paneling) & \(634.99 \mathrm{sf}(\mathrm{cad})\) \\
beam seat width \(=\) & 6.75 ft \\
lower beam seat area \(=\) & \(156.94 \mathrm{sf}(\mathrm{cad})\) \\
lower beam seat width \(=\) & 5.50 ft \\
Beam Seat Volume & 5149.34 cf
\end{tabular}
\begin{tabular}{lr} 
backwall area \(=\) & \(682.99 \mathrm{sf}(\mathrm{cad})\) \\
backwall thickness \(=\) & 1.75 ft \\
approach slab thickness \(=\) & 1.25 ft \\
abutment length \(=\) & 156.94 ft \\
Back Wall Volume \(=\) & 1097.14 cf \\
Additonal Volume & \\
pedestals = & 16.92 cf \\
end wall volumes = & 191.79 cf \\
Rear Abutment Subtotal & 239 cy
\end{tabular}
```


## Forward Abutment:

```
Beam Seat
beam seat area (above paneling) 527.19 sf (cad)
beam seat width = lower beam seat area = lower beam seat width = Beam Seat Volume =6.75 ft
139.76
                            5.50 ft
4327.21 cf
Backwall
backwall area \(=\quad 673.50\) sf (cad)
backwall thickness = 1.75 ft
approach slab thickness = 1.25 ft
abutment length = 151.77 ft
Back Wall Volume = 1083.77 cf
Additonal Volume
pedestals \(=\quad 16.92\) cf
end wall volumes = 105.26 cf
Forward Abutment Subtotal = 205 cy
```



## ITEM 511 - CLASS QC1 CONCRETE WITH QC/QA, FOOTING, AS PER PLAN

```
Pier:
\begin{tabular}{lr} 
height \(=\) & 3 ft \\
width \(=\) & 15 ft \\
length & 147.66 ft
\end{tabular}
length \(=\quad 147.66 \mathrm{ft}\)
Total \(=\underline{\mathbf{2 4 7}}\) cy
```


## Sidewalk on BRIDGE:

| area $=$ | 6042.46 sf |
| :--- | ---: |
| avg. thickness $=$ | 8.25 in |
| Sidewalk Volume $=$ | 154 cy |

Sidewalk on CAP (sidewalk portion over end sections only):

| area $=$ | 1526.61 sf |
| :--- | ---: |
| avg. thickness $=$ | 8.25 in |
| Sidewalk Volume $=$ | 39 cy |

Total $=\quad \underline{193}$ cy

## ITEM 512 - SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)

| Rear Abutment: | Face | 2967.00 sf |
| :--- | :--- | ---: |
|  | Top | 863.17 sf |
| Forward Abutment : | Face | 4546.72 sf |
| Pier : | Top | 1012.00 sf |
|  | Face | 2649.34 sf |
|  |  |  |
|  | Total $=$ |  |
|  | $\mathbf{1 , 3 3 8} \mathrm{sy}$ |  |

## ITEM 512-TYPE 2 WATERPROOFING

| Rear Abutment $=$ | 9.68 ft |
| :--- | :--- |
| Forward Abutment $=$ | 8.47 ft |

Total $=\quad \underline{7}$ sy

## ITEM 512 - SEALING OF CONCRETE SURFACES (NON-EPOXY)

| west parapet $=$ | 906.90 sf |
| :--- | :--- |
| east parapet $=$ | 1155.07 sf |
| pylons $=$ | 451.87 sf |
| west sidewalk $=$ | 2906.80 sf (includes small area of sidewalk on SW abutment) |
| east sidewalk $=$ | 2388.40 sf |
| east cap $=$ | $3539.28 \mathrm{sf}=====>$ cap total $=$ |
|  | Total $=\quad$ 394 sy |
|  | 1261 sy |

## ITEM 513 - STRUCTURAL STEEL MEMBERS, LEVEL 4

| BRIDGE |  |  |  |
| :---: | :---: | :---: | :---: |
| Girders: |  |  |  |
| Section 1 |  |  |  |
| length = | 65.50 ft |  |  |
| weight $=$ | $225.43 \mathrm{lbs} / \mathrm{ft}$ |  |  |
| Section 2 |  |  |  |
| volume $=$ | 43.53 cf |  |  |
| unit weight steel $=$ | 21327.4 lbs |  |  |
| Section 3 |  |  |  |
| length $=$ | 67.25 ft |  |  |
| weight $=$ | $225.43 \mathrm{lbs} / \mathrm{ft}$ |  |  |
| Detail factor $=$ | 1.0 |  |  |
| No. of beams = | 10 |  |  |
| Beam Subtotal $=$ | 512,600 lbs |  |  |
| Splices: | \# of plates length (in) | width (in) | thick (in) |
| Top Flange |  |  |  |
| outside plates = | 137 | 18 | 0.625 |


| inside plates = | 2 | 37 | 8 | 0.625 |
| :---: | :---: | :---: | :---: | :---: |
| Bottom Flange |  |  |  |  |
| outside plates = | 1 | 44 | 18 | 0.75 |
| inside plates = | 2 | 44 | 8 | 0.75 |
| Web |  |  |  |  |
| plates $=$ | 2 | 29 | 22.5 | 0.5 |
| Plate weight/splice = | 726 lbs |  |  |  |
| Splice Bolts | \# of bolts | length (in) | bolt wt. | washer wt. |
| Top Flange = | 40 | 1.5 | 148 | 11.3 |
| Bot. Flange = | 48 | 1.5 | 148 | 11.3 |
| Web = | 48 | 1.5 | 148 | 11.3 |
|  | * from steel * washer w | manual eight is per | $0 \mathrm{ct}$. |  |


| Bolt + Washer weight |  |
| :--- | :---: |
| Top Flange $=$ | 64 lbs |
| Bot. Flange $=$ | 76 lbs |
| Web $=$ | 76 lbs |
|  |  |
| Total weight/splice $=$ | 943 lbs |
| No. of splices $=$ | 20 |

Splice + Bolts Subtotal $=\quad 18,856$ lbs

## Intermediate Crossframes:

| Length of Diagonals = | 10.67 ft (weighted avg.) |
| :---: | :---: |
| No. of Diagonals = | 2 |
| Length of Horiz. = | 10.62 ft (weighted avg.) |
| No. of Horiz. = | 1 |
| Angle weight $/ \mathrm{ft}$. $=$ | $9.80 \mathrm{lbs} / \mathrm{ft}$ |
| Crossframe weight $=$ | $313 \mathrm{lbs}==>$ per x -frame assembly |
| x-frame stiffeners? | y y or n |
| Length = | 34.000 in ==> web depth |
| Width = | 5.000 in |
| Thickness = | 0.375 in |
| Stiffener weight = | $36 \mathrm{lbs}==>$ per x -frame assembly |
| Total Intermediate Crossframe |  |
| Assembly Weight = | 349 lbs. ==> per $x$-frame assembly |
| No. of assemblies = | 145 |

## End Crossframes:

| Length of Diagonals $=$ | 10.85 ft (weighted avg.) |
| :--- | :---: |
| No. of Diagonals = | 2 |
| Length of Horiz. $=$ | 10.78 ft (weighted avg.) |
| No. of Horiz. $=$ | 1 |
| Angle weight $/ \mathrm{ft}=$ | $9.80 \mathrm{lbs} / \mathrm{ft}$ |
|  |  |
| Crossframe weight = | $318 \mathrm{lbs} .==>$ per x-frame assembly |
|  |  |
| x-frame stiffeners? | y y or n |
| Length = | $5.000 \mathrm{in}==>$ web depth |
| Width = | 34.000 in |
| Thickness = | 0.375 in |
| Stiffener weight = | $36 \mathrm{lbs}==>$ per x-frame assembly |

Total x-frame End
Assembly Weight $=\quad 354 \mathrm{lbs}==>$ per x -frame assembly
No. of assemblies = 18
Crossframe Subtotal $=\quad 57,038 \mathrm{Ibs}$

## Signal Support Diaphragm:

Embedded steel plate (20"x20"x1.75") = 198 lbs
Support angles (L6x4×1/2) - Length =
Support angle weight per $\mathrm{ft}=$ No. of support angles = 10.833 ft $16.2 \mathrm{lb} / \mathrm{ft}$

No. of signal support diaphragms = 3 ea

Signal support subtotal $=$ 1,648 lbs

BRIDGE STRUCTURAL STEEL =
590,142 lbs

## CAP

Girders:
Section 1

| length = | 65.50 ft |
| :--- | :---: |
| weight = | $202.47 \mathrm{lbs} / \mathrm{ft}$ |
| Section 2 |  |
| volume $=$ | 42.49 cf |
| unit weight steel $=$ <br> Section 3 | 20820 lbs |
| length = |  |
| weight = | 67.25 ft |
| Detail factor = | $202.47 \mathrm{lbs} / \mathrm{ft}$ |
| No. of beams = | 1.0 |
|  | 5 |


| Beam subtotal $=$ | 238,500 lbs |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| Splices: | \# of plates | length (in) | width (in) | thick (in) |
| Top Flange <br> outside plates $=$ | 1 | 37 | 18 | 0.625 |
| inside plates $=$ <br> Bottom Flange | 2 | 37 | 8 | 0.625 |
| outside plates $=$ <br> inside plates $=$ | 1 | 44 | 18 | 0.75 |
| Web <br> plates $=$ | 2 | 44 | 8 | 0.75 |
| Plate weight/splice $=$ 2 29 22.5 0.5 | 726 lbs |  |  |  |


| Splices Bolts | \# of bolts | length (in) | bolt wt. | washer wt. |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Top Flange $=$ | 40 | 1.5 | 148 | 11.3 |  |
| Bot. Flange $=$ | 48 | 1.5 | 148 | 11.3 |  |
| Web $=$ | 48 | 1.5 | 148 | 11.3 |  |
|  | * from steel manual |  |  |  |  |
|  |  |  |  |  |  |
|  | * washer weight is per 100 ct. |  |  |  |  |


| Bolt + Washer weight |  |
| :--- | :---: |
| Top Flange $=$ | 64 lbs |
| Bot. Flange $=$ | 76 lbs |
| Web $=$ | 76 lbs |
|  |  |
| Total weight/splice $=$ | 943 lbs |
| No. of splices $=$ | 10 |

Splice subtotal $=\quad 9,428 \mathrm{lbs}==>$ plates + bolts

Intermediate Crossframes:

| Length of Diagonals = | 8.91 ft (weighted avg.) |
| :---: | :---: |
| No. of Diagonals = | 2 |
| Length of Horiz. = | 8.82 ft (weighted avg.) |
| No. of Horiz. = | 1 |
| Angle weight $/ \mathrm{ft}$. $=$ | $9.80 \mathrm{lbs} / \mathrm{ft}$ |
| Crossframe weight $=$ | 261 lbs ==> per $x$-frame assembly |
| x-frame stiffeners? | y y or n |
| Length = | 34.000 in ==> web depth |
| Width = | 5.000 in |
| Thickness = | 0.375 in |
| Stiffener weight $=$ | $36 \mathrm{lbs}==>$ per x -frame assembly |
| Total $x$-frame assembly weight = No. of assemblies = | 297 lbs ==> per x-frame assembly 65 |
| End Crossframes: |  |
| Length of Diagonals = | 9.00 ft (weighted avg.) |
| No. of Diagonals = | 2 |
| Length of Horiz. = | 8.90 ft (weighted avg.) |
| No. of Horiz. = | 1 |
| Angle weight / ft. = | $9.80 \mathrm{lbs} / \mathrm{ft}$ |
| Crossframe weight $=$ | 264 lbs ==> per $x$-frame assembly |
| x-frame stiffeners? | y y or n |
| Length $=$ | 34.000 in ==> web depth |
| Width = | 5.000 in |
| Thickness = | 0.375 in |
| Stiffener weight $=$ | $36 \mathrm{lbs}==>$ per x -frame assembly |
| Total x -frame |  |
| assembly weight = | $300 \mathrm{lbs}==>$ per $x$-frame assembly |
| No. of assemblies = | 8 |

## Crossframe Subtotal $=\quad 21,715 \mathrm{Ibs}$

## Signal Support Diaphragm:

Embedded steel plate (20"x20"x1.75") = 198 lbs
Support angles (L6x4×1/2) - Length $=\quad 9.000 \mathrm{ft}$
Support angle weight per $\mathrm{ft}=\quad 16.2 \mathrm{lb} / \mathrm{ft}$
No. of support angles = 2 ea
No. of signal support diaphragms $=\quad 1$ ea
Signal support subtotal $=\quad 490 \mathrm{lbs}$
CAP STRUCTURAL STEEL = 270,133 lbs

Total $=860,275 \mathrm{lbs}$

```
    Bridge No. rows per beam = 259
    No. per row = 3
    No. of beams = 10
    Number per signal support = 50
    Number of signal supports = 3
    Bridge Total = 7920 ea
    Cap No. rows per beam= 277
    No.per row = 3
    No. of beams = 5
    Number per signal support = 50
    Number of signal supports = 1
    Bridge Total = 4205 ea
                            Total = 12,125 ea
ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT
ITEM 514 - FIELD PAINTING STRUCTURAL STEEL, FINISH COAT
```



## ITEM 514 - FINAL INSPECTION REPAIR

| Length $=$ | 198.75 ft |
| :--- | ---: |
| No. Girders $=$ | 15 ea |
| No. Crossframes $=$ | 236 ea |

Total $=\quad \underline{32}$ ea

## ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL (3")

```
Length = 294.00 ft
    Total = \underline{294 ft}
```

ITEM 516 -STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL (5")
Length $=\quad 198.82 \mathrm{ft}$

## ITEM 516-1" PREFORMED EXPANSION JOINT FILLER

## Rear Abutment:

| beam seat height = | 5.33 ft |
| :--- | ---: |
| beam seat width = | 6.75 ft |
| beam seat length = | 156.94 ft |
| wall height = | 17.65 ft |
| wall width = | 0.83 ft |
| backwall height = | 4.31 ft |
| backwall width = | 1.75 ft |

Forward Abutment:

| beam seat height = | 8.46 ft |
| :--- | ---: |
| beam seat width = | 6.75 ft |
| beam seat length = | 151.77 ft |
| wall height = | 20.55 ft |
| wall width = | 0.83 ft |
| backwall height = | 4.44 ft |
| backwall width = | 1.75 ft |

Total $=\quad \underline{398} \mathbf{~ s f}$

## ITEM 516-2" PREFORMED EXPANSION JOINT FILLER

Pier:
height $=\quad 24.24 \mathrm{ft}$
Total $=\quad \underline{73} \mathbf{~ s f}$
ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES \& LOAD PLATE (NEOPRENE) 10.5" x 1'-4" x 2.45" PAD WITH 11.5" x 1'-10" BEVELED PLATE, AS PER PLAN

| Rear Abutment $=$ | 15 ea |
| :--- | :--- |
| Forward Abutment $=$ | 15 ea |

Total $=\quad \underline{30}$ ea
ITEM 516 - ELASTOMERIC BEARING WITH INTERNAL LAMINATES \& LOAD PLATE (NEOPRENE) 1'-6" x 2'-0" x 3.40" PAD WITH 1'-7" x 2'-9" BEVELED PLATE, AS PER PLAN

Pier $=\quad 15 \mathrm{ea}$
Total $=\quad \underline{15}$ ea

## ITEM 518 - POROUS BACKFILL WITH GEOTEXTILE FABRIC

Rear Abutment:

| avg. top of backwall $=$ | 753.73 |
| :--- | ---: |
| top of drilled shaft $=$ | 744.45 |
| height $=$ | 7.53 ft |
| length $=$ | 156.94 ft |
| thickness $=$ | 2.00 ft |
| area (panel footing $)=$ | 6.58 sf |

3,397 cf

Forward Abutment:
avg. top of backwall $=757.48$
top of drilled shaft $=\quad 748.35$
height $=$ 7.38 ft
length $=\quad 151.77 \mathrm{ft}$ thickness $=\quad 2.00 \mathrm{ft}$
area $($ panel footing $)=6.58 \mathrm{sf}$ Forward Abutment Subtotal $=\quad 3,238$ cf

Total $=\quad \underline{246} \mathbf{c y}$

## ITEM 518-6" PERFORATED CORRUGATED PLASTIC PIPE

## Rear Abutment:

length $=\quad 313.88 \mathrm{ft} \quad$ (inlcudes façade panel pipe)

Forward Abutment:
length $=303.54 \mathrm{ft} \quad$ (inlcudes façade panel pipe)

Total $=\quad \underline{618} \mathrm{ft}$

## ITEM 518-6" NON-PERFORATED CORRUGATED PLASTIC PIPE

Rear Abutment:
length =
20 ft
Forward Abutment:
length =
27 ft
Total $=\quad \underline{47} \mathrm{ft}$

## ITEM 524 - DRILLED SHAFTS, 60" DIAMETER, ABOVE BEDROCK

Rear Abutment:

| length $=$ | 54.45 ft |
| :--- | :---: |
| no. of columns $=$ | 31 |

Forward Abutment:

| length $=$ | 58.35 ft |
| :--- | :---: |
| no. of columns $=$ | 45 |

## ITEM 526 - REINFORCED CONCRETE APPROACH SLABS WITH QC/QA (T=15"), AS PER PLAN

Rear Approach Slab:
area $=\quad 3957.64 \mathrm{sf}$
Forward Approach Slab:

```
area = 4656.49 sf
```

Total $=\quad \underline{958}$ sy

## ITEM SPECIAL - STRUCTURE, MISC.: PERMANENT UTILITY SUPPORTS

| Length of Horiz. = | 10.78 ft (weighted avg.) |
| :---: | :---: |
| No. of Horiz. = | 1 |
| Angle weight $/ \mathrm{ft}$. $=$ | $12.80 \mathrm{lbs} / \mathrm{ft}$ |
| utility support weight = | 138 lbs ==> per assembly |
| Stiffeners? | y y or n |
| Length = | 5.000 in ==> web depth |
| Width = | 34.000 in |
| Thickness = | 0.375 in |
| Stiffener weight $=$ | $36 \mathrm{lbs}==>$ per x -frame assembly |
| Total utility support |  |
| assembly weight = | 174 lbs ==> per x-frame assembly |
| No. of assemblies = | 29 |

।
Total Weight $=\quad 5050 \mathrm{lbs}$
Cost $=\quad \$ 1.50$ per lb
Total $=\$ 7,576.00$

## ITEM SPECIAL - STRUCTURE, MISC.: PRECAST FACADE PANELS

Rear Abutment:
area $=\quad 2769.95 \mathrm{sf}$

Forward Abutment:
area $=\quad 3261.59 \mathrm{sf}$
Total $=\quad \underline{6,032} \mathbf{s i}$

ITEM 607 - FENCE, MISC.: WALL MOUNTED TYPE A (W/ VANDAL MESH)
Rear Abutment:
length $=\quad 4.5 \mathrm{ft}==>$ on SW knee wall extension
North end of west parapet:
length =
$15.50 \mathrm{ft}==>$ north of NW end pilaster
Total $=\quad \underline{20} \mathrm{ft}$
ITEM 625 - LIGHT POLE ANCHOR BOLTS, MISC.: COMBINATION SIGNAL POLE AND PEDESTRIAN POLE ANCHOR BOLT ASSEMBLIES EMBEDDED IN CONCRETE BRIDGE DECK
Total $=\quad \underline{8}$ ea

