



CHA COMPUTATION PAD

COMPLETED BY: CAE

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PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

74567

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SHEET #: 1 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Summary Report

ITEM	DESCRIPTION	QUANTITY	UNIT	TYPE
202E11203	PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN	1.00	LS	S
202E22900	APPROACH SLAB REMOVED	167.00	SY	S
503E21100	UNCLASSIFIED EXCAVATION	329.00	CY	S
509E10000	EPOXY COATED REINFORCING STEEL	209,334.00	LB	M
509E20001	CONCRETE REINFORCEMENT, REPLACEMENT OF EXISTING CONCRETE REINFORCEMENT, AS PER PLAN	1,000.00	LB	U
509E25000	UNCOATED STEEL REINFORCEMENT	1,600.00	LB	U
509E30020	NO. 4 DEFORMED GRFP REINFORCEMENT	13,528.00	FT	U
510E10001	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT, AS PER PLAN	216.00	EACH	M
511E34446	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK	525.00	CY	R
511E34451	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET), AS PER PLAN	137.00	CY	U
511E40512	CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS	177.00	CY	S
511E44112	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING	83.00	CY	S
511E46012	CLASS QC1 CONCRETE WITH QC/QA, RETAINING/WINGWALL NOT INCLUDING FOOTING	43.00	CY	S
512E10100	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	1,553.00	SY	S
512E33000	TYPE 2 WATERPROOFING	30.00	SY	S
513E10240	STRUCTURAL STEEL MEMBERS, LEVEL 2	550,447.00	LB	U
513E20000	WELDED STUD SHEAR CONNECTORS	9,400.00	EACH	S
514E00060	FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT	31,801.00	SF	U
514E00066	FIELD PAINTING STRUCTURAL STEEL, FINISH COAT	31,801.00	SF	U
516E11210	STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL	85.00	FT	U
516E13900	2" PREFORMED EXPANSION JOINT FILLER	79.00	SF	S
516E44001	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (FIXED)	5.00	EACH	R
516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	30.00	EACH	R
518E21200	POROUS BACKFILL WITH GEOTEXTILE FABRIC	89.00	CY	S
518E40000	6" PERFORATED CORRUGATED PLASTIC PIPE	114.00	FT	S
518E40010	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	102.00	FT	S
526E25001	REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN	209.00	SY	S
526E90010	TYPE A INSTALLATION	83.00	FT	S
601E20000	CRUSHED AGGREGATE SLOPE PROTECTION	642.00	SY	U
607E39901	VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC, AS PER PLAN	742.00	FT	S
625E33001	STRUCTURE GROUNDING SYSTEM, AS PER PLAN	1.00	EACH	S
846E00111	POLYMER MODIFIED ASPHALT EXPANSION JOINT SYSTEM, AS PER PLAN	35.00	CF	



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SHEET #: 2 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

202E11203	PORTIONS OF STRUCTURE REMOVED, OVER 20 FOOT SPAN, AS PER PLAN	TOTAL:	1.00	LS
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Based on Deck Area

Total Deck Area = 17858.0 sf Total = \$813,102

\$/sf = \$45.5



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SHEET #: 3 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

202E22900	APPROACH SLAB REMOVED	TOTAL:	167.00	SY
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Forward Approach Slab Area = 751 sf

Rear Approach Slab Area = 750.86 sf

Total = 1501.9 sf

= 167 sy



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SHEET #: 4 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

503E21100	UNCLASSIFIED EXCAVATION	TOTAL:	329.00	CY
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a. Behind Abutment

Depth of Excavation = 9.22 ft

Additional Length to Excavation = 1.5 ft

Abutment Width = 39.255 ft

Excavation Cross-Sectional Area = 56.326 sft

Volume per Abutment = 2211.1 cft

= 81.892 cy

b. In Front of Abutment

Assumed Depth of Excavation = 3 ft

Length of Excavation = 43.255 ft

Excavation Cross-Sectional Area = 4.5 sft

Volume per Abutment = 194.65 cft

= 7.2092 cy

c. Behind and Infront of wingwall

Length of Wingwall = 17 ft

Wingwall Height = 9.2192 ft

Additional Length to Excavation = 1.5 ft

Grand total = 329 cy

Excavation Cross-Sectional End Area = 56.326 sft

Volume per Wingwall = 319.18 cft

= 11.822 cy

(This volume is assumed to lap with the volume behind the abutment. So, this overlap deals with the soil on the outside.

(Width of Foundation * Length of Foundation * Assumed Depth) - (Volume of Pier Column) = Volume

= 185.86 cft Per Column

Number of Columns = 15 Total = 2787.9 cft



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SHEET #: 5 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

509E10000	EPOXY COATED REINFORCING STEEL	TOTAL:	209,334.00	LB
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Deck	143777.00	lbs
Abutment and Wingwall	12238.00	lbs
Pier	38042.00	lbs
Railing	15277.00	lbs

Total = 209334.00 lbs



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SHEET #: 6 OF 38

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SUBJECT: Quantity Calculations

509E20001	CONCRETE REINFORCEMENT, REPLACEMENT OF EXISTING CONCRETE REINFORCEMENT, AS PER PLAN	TOTAL:	1,000.00	LB
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Total from general notes = 1000 lbs



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SHEET #: 7 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

509E25000	UNCOATED STEEL REINFORCEMENT	TOTAL:	1,600.00	LB
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A518 BAR = 245 LB

A604 BAR = 1298 LB

A527 BAR = 44 LB

A528 BAR = 13 LB

TOTAL = 1600 LB



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SHEET #: 8 OF 38

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509E30020	NO. 4 DEFORMED GRFP REINFORCEMENT	TOTAL:	13,528.00	FT
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From Rebar Calcs = 13528 ft



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SHEET #: 9 OF 38

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510E10001	DOWEL HOLES WITH NONSHRINK, NONMETALLIC GROUT, AS PER PLAN	TOTAL:	216.00	EACH
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Number of A518 bars = 54

Number of A604 bars = 162

Total Number of Dowels = 216



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SHEET #: 10 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

511E34446	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK	TOTAL:	525.00	CY
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Interior Deck

$$\begin{aligned}\text{Bridge Deck Thickness} &= 8.50 \text{ in} = 0.71 \text{ ft} \\ \text{Width} &= 28.68 \text{ ft} \\ \text{Bridge Floor Length} &= 490.00 \text{ ft} \\ \text{Total} &= 490.00 \times 28.68 \times 0.71 = 9955.51 \text{ cft}\end{aligned}$$

Overhang Deck

$$\begin{aligned}\text{Bridge Deck Thickness} &= 11.875 \text{ in} = 0.99 \text{ ft} \\ \text{Total Width} &= 7.98 \text{ ft} \\ \text{Bridge Floor Length} &= 490.00 \text{ ft} \\ \text{Total} &= 490.00 \times 7.98 \times 0.99 = 3871.05 \text{ cft}\end{aligned}$$

Haunch

$$\begin{aligned}\text{Average Haunch Thickness} &= 2.126 \text{ in} = 0.18 \text{ ft} \\ \text{Haunch Width} &= 15.80 \text{ in} = 1.32 \text{ ft} \\ \text{Span 1 Beam Length} &= 66.05 \text{ ft} \\ \text{Span 2 Beam Length} &= 92.00 \text{ ft} \\ \text{Span 3 Beam Length} &= 84.00 \text{ ft} \\ \text{Span 4 Beam Length} &= 84.00 \text{ ft} \\ \text{Span 5 Beam Length} &= 95.25 \text{ ft} \\ \text{Span 6 Beam Length} &= 69.05 \text{ ft} \\ \text{Total Length} &= 490.35 \text{ ft} \\ \text{No. of Beams per Span} &= 3.00 \\ \text{Total} &= 490.35 \times 1.32 \times 0.18 \times 3.00 = 343.22 \text{ cft}\end{aligned}$$

$$\begin{aligned}\text{Total Volume} &= 9955.51 \text{ cft} + 3871.05 \text{ cft} + 343.22 \text{ cft} \\ &= 14169.78 \text{ cft} \\ &= 525.00 \text{ cy}\end{aligned}$$



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511E34451	CLASS QC2 CONCRETE WITH QC/QA, BRIDGE DECK (PARAPET), AS PER	TOTAL:	137.00	CY
	PLAN			

SBR-3 Concrete Rail

$$\begin{aligned}\text{Concrete Height} &= 3.00 \text{ ft} \\ \text{Concrete Top Width} &= 0.83 \text{ ft} \\ \text{Concrete Bottom Width} &= 1.50 \text{ ft} \\ \text{Rail Length} &= 490.00 \text{ ft} + 490.00 \text{ ft} + 16.6145 \text{ ft} = 996.61 \text{ ft} \\ &\text{*Extra at Ends on Approach Slab} \\ \text{Total} &= 3488.15 \text{ cft} = 129.19 \text{ cys}\end{aligned}$$

SBR-3 Transition

$$\begin{aligned}\text{Concrete Volume} &= 1.74 \text{ cys} \\ \text{No. of Transition} &= 3.00 \\ \text{Total} &= 5.22 \text{ cys}\end{aligned}$$

(Standard Bridge Drawing SBR-3-20)

Impact Attenuator Transition

$$\begin{aligned}\text{Transition Volume} &= 1.74 \text{ cys} \\ \text{SBR-3 C to E Volume} &= 0.46 \text{ cys} \\ \text{Concrete Block Volume} &= 0.48 \text{ cys} \\ \text{Combined} &= 1.76 \text{ cys}\end{aligned}$$

Average end area for sections between L and N on Plans

Transition - C to E + Block

$$\begin{aligned}\text{Total Volume} &= 129.19 \text{ cys} + 5.22 \text{ cys} + 1.76 \text{ cys} \\ &= 137.00 \text{ cys}\end{aligned}$$



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SHEET #: 12 OF 38

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SUBJECT: Quantity Calculations

511E40512	CLASS QC1 CONCRETE WITH QC/QA, PIER ABOVE FOOTINGS	TOTAL:	177.00	CY
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Columns

Pier 1 Height = 17.96 ft (ELEV F - ELEV G + 3')

Pier 2 Height = 18.66 ft

Pier 3 Height = 20.00 ft

Pier 4 Height = 18.31 ft

Pier 5 Height = 17.09 ft

Column Width = 3.5 ft

Volume = 98.4 cy

Pier Cap

Pier 1 Cap Height = 3.16 ft

Pier 2 Cap Height = 3.19 ft

Pier 3 Cap Height = 3.22 ft

Pier 4 Cap Height = 3.05 ft

Pier 5 Cap Height = 3.28 ft

Top Area = 14.81 sy

Volume = 78.5 cy

Total = 177 cy



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SHEET #: 13 OF 38

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SUBJECT: Quantity Calculations

511E44112	CLASS QC1 CONCRETE WITH QC/QA, ABUTMENT NOT INCLUDING FOOTING	TOTAL:	83.00	CY
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Rear Abutment

Average Abutment Height = 9.08 ft

Average Stem Height = 4.75 ft

Average Backwall Height = 4.32 ft

Corner Backwall

Length = 2.50 ft (Each Side)

Volume = 75.057 cf

Backwall Area = 6.94 sft

Stem Area = 18.828 sft

Corner Fillet

Top area = 2.33 sf (Measured in CAD)

Height = 2.57 ft (6" below app slab seat)

Length of Abutment = 39.255 ft

Volume = 6.0 cf

Volume = 1092.7 cft

= 40.5 cy

Forward Abutment

Average Abutment Height = 9.36 ft

Average Stem Height = 5.05 ft

Average Backwall Height = 4.31 ft

Corner Backwall

Length = 2.50 ft (Each Side)

Volume = 77.529 cf

Backwall Area = 6.91 sft

Stem Area = 20.03 sft

Corner Fillet

Top area = 2.33 sf (Measured in CAD)

Height = 2.56 ft (6" below app slab seat)

Length of Abutment = 39.255 ft

Volume = 6.0 cf

Volume = 1141.0 cft

= 42.3 cy



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SHEET #: 14 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

511E46012	CLASS QC1 CONCRETE WITH QC/QA, RETAINING/WINGWALL NOT INCLUDING FOOTING	TOTAL:	43.00	CY
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Wingwall

Side Area

Rear Left = 128.28 sf

Rear Right = 124.53 sf

Forward Left = 123.87 sf

Forward Right = 128.5 sf

Block Area

Rear Left = 46.706 sf

Rear Right = 43.103 sf

Forward Left = 43.772 sf

Forward Right = 52.689 sf

Side Thickness = 1.5 ft

Block Thickness = 2 ft

Fillet Volumes

Rear Left = 11.096 cf

Forward Right = 11.796 cf

Volume per Wingwall

Rear Left = 296.92 cf

Rear Right = 273.01 cf

Forward Left = 273.34 cf

Forward Right = 309.93 cf

Total = 1153.2 cf

= 43 cy



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SHEET #: 15 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

512E10100	SEALING OF CONCRETE SURFACES (EPOXY-URETHANE)	TOTAL:	1,553.00	SY
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Sealing of Railing on Bridge

Height of Railing = 3 ft

Slanted Face = 3.0542 ft

Top of Railing = 0.9271 ft

Deck Thickness = 0.9167 ft

Deck Lip = 0.1667 ft

Under Deck Limit = 0.5 ft

Sum = 8.5646 ft

Total Length of Railing = 980.33 ft

Subtotal = 8396.2 sf

Sealing of Abutments

Height of slope below Beam Seat = 1 ft

Beam Seat Width = 2.25 ft

Backwall Height - Joint = 3.9067 ft

Length of Beamseat = 39.167 ft

Area of Abutment on Beamseat = 280.3 sf

Length of Side of Beamseat = 5.2547 ft

Area of Beamseat Sides = 5.2547 sf

Length of Rest of Abutment = 3.8272 ft

Area of Rest of Abutment = 18.779 sf

Area of Single Abutment = 304.34 sf

Number of Abutments = 2

Subtotal = 608.67 sf

Total = 13975 sf

= 1553 sy

Sealing of Railing for BTA (Conservative)

Height of Railing = 3 ft

Slanted Face = 3.0542 ft

Top of Railing = 0.9271 ft

Deck Lip = 0.1667 ft

Sum = 7.148 ft

Total Length of Railing BTA = 53.313 ft

Impact Block Railing Total = 175.79 sf

Subtotal = 556.87 sf

Sealing of Wingwalls

Average Height = 2.745 ft (Beamseat to 0)

Length of Wingwall = 17 ft

Odd Wingwall = 15.917 ft

Side Area of Wingwalls = 183.69 sf

Width Face of Wingwall = 1.5 ft

Height of Abut. and Below = 5.49 ft

Number of Wingwalls = 4

Face of Abutment Area = 32.94 sf

Subtotal = 216.63 sf

Sealing of Piers

Average Column Height = 18.40 ft

Average Soil Height = 4.00 ft (Conservative)

Circumference of Column = 10.996 ft

Pier Cap Perimeter = 81.662 ft

Average Pier Cap Height = 3.18 ft

Top Area = 133.29 sf

Bottom Area = 104.42 sf

Subtotal = 4196.7 sf



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SUBJECT: Quantity Calculations

512E33000	TYPE 2 WATERPROOFING	TOTAL:	30.00	SY
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Height = 3 ft

Width of abutment backwall = 43.573 ft

One Abutment = 130.72 ft

Total = 261.44 ft

= 29.049 sy



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DATE: 3/4/2025

SUBJECT: Quantity Calculations

513E10240 STRUCTURAL STEEL MEMBERS, LEVEL 2

TOTAL:

550,447

LB

Beam

				Beam Used	Weight (lbs/ft)	Beam Weight (lbs)
(Add 0.5 ft on end)	Span 1	=	66.05 ft	W40x199	199	13144
	Span 2	=	92.00 ft	W40x199	199	18308
	Span 3	=	84.00 ft	W40x199	199	16716
	Span 4	=	84.00 ft	W40x199	199	16716
	Span 5	=	95.25 ft	W40x199	199	18955
(Add 0.5 ft on end)	Span 6	=	69.05 ft	W40x199	199	13741
Total Weight =						97580 lbs

No. of Beams Lines = 5.00

Weight of Steel Beam = 97580 ft x 5.00
= 487902.4 lb

Cross Frame

Steel Section = L3.5x3.5x3/8
Weight per linear foot = 8.5 lb/ft
Length of Cross Frames = 29.412 ft
Total Number of Cross Frames = 72.00

Strut

Steel Section = L3.5x3.5x3/8
Weight per linear foot = 8.5 lb/ft
Length of Strut = 14.538 ft
Total Number of Struts = 62.00

Skewed Crossframe

Steel Section = L4x4x1/2
Weight per linear foot = 12.80 lb/ft
Number of Skewed Cross Frames = 20.00
Assumed Length Increase = 34.64

End Diaphragm

Steel Section = L4x4x3/8
Weight per linear foot = 9.80 lb/ft
Length of Angles = 23.01
Number of Skewed Cross Frames = 8.00

Total Weight of Framing = 29.412 ft x 8.5 lb/ft x 72.00
+ 14.538 ft x 8.5 lb/ft x 62.00
+ 23.006 ft x 9.8 lb/ft x 8.00
+ 34.64 ft x 12.80 lb/ft x 20.00
= 36332.0 lb

Adding 5% for connections and splice

Total weight = 550,447 lb



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513E20000	WELDED STUD SHEAR CONNECTORS	TOTAL:	9,400.00	EACH
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Shear stud connector per beam line = 1880 ea. (From Shear Stud Calculations)

Total Beam Lines = 5

Total = 9400 ea.



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SHEET #: 19 OF 38

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514E00060	FIELD PAINTING STRUCTURAL STEEL, INTERMEDIATE COAT	TOTAL:	31,801.00	SF
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Beam

For W40x199

h = 38.70 in

w = 15.80 in

tw = 0.65 in

tf = 1.07 in

*Painting region = 10.29 ft

*(top flange width not considered for painting)

Span 1 Beam Length = 66.05 ft

Span 2 Beam Length = 92.00 ft

Span 3 Beam Length = 84.00 ft

Span 4 Beam Length = 84.00 ft

Span 5 Beam Length = 95.25 ft

Span 6 Beam Length = 69.05 ft

Total Length = 490.354 ft

No. of Beams per Span = 5.00

Surface Area = 10.29 ft x 490.35 ft x 5.00

= 25232.8 sf

Framing

For L3.5x3.5x3/8

h = 3.50 in

w = 3.50 in

tw = 0.38 in

tf = 0.38 in

Painting region = 1.17 ft

Length = 3019.020 ft

Surface Area = 3522.19 sf

For L4x4x3/8

h = 4.0 in

w = 4.00 in

tw = 0.38 in

tf = 0.38 in

Painting region = 1.33 ft

Length = 184.048 ft

= 245.40 sf

For L4x4x1/2

h = 4.0 in

w = 4.00 in

tw = 0.50 in

tf = 0.50 in

Painting region = 1.33 ft

Length = 692.711 ft

= 923.62 sf

Adding 40% to Framing for connections plates and Splice

Total Surface Area = 31,801.00 sf



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SHEET #: 20 OF 38

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514E00066	FIELD PAINTING STRUCTURAL STEEL, FINISH COAT	TOTAL:	31,801.00	EACH
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SHEET #: 21 OF 38

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516E11210	STRUCTURAL EXPANSION JOINT INCLUDING ELASTOMERIC STRIP SEAL	TOTAL:	85.00	FT
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Forward Abut = 42.281 ft

Rear Abut = 42.281 ft



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DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E13900	2" PREFORMED EXPANSION JOINT FILLER	TOTAL:	79.00	SF
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Wingwall and Approach Slab Lengths

Northwest = 15.748 ft

Southwest = 15.269 ft

Northeast = 15.269 ft

Southeast = 16.54 ft

Sum = 62.823 ft

Height = 1.25 ft (Approach slab thickness)

Total = 79 sf



CHA COMPUTATION PAD

COMPLETED BY: CAE

PROJECT	PHASE	ORG
74567	0	0

CHECKED BY: DOR

SHEET #: 23 OF 38

PROJECT NAME: LAK-6-2.06

DATE: 3/4/2025

PROJECT LOCATION: Willoughby Hills (LAK County)

SUBJECT: Quantity Calculations

516E44001	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (FIXED)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 24 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

PROJECT	PHASE	ORG
74567	0	0

CHECKED BY: DOR

SHEET #: 25 OF 38

PROJECT NAME: LAK-6-2.06

DATE: 3/4/2025

PROJECT LOCATION: Willoughby Hills (LAK County)

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 26 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

74567

PHASE

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ORG

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SHEET #: 27 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 28 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 29 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

516E44201	ELASTOMERIC BEARING WITH INTERNAL LAMINATES AND LOAD PLATE (NEOPRENE), AS PER PLAN (EXPANSION)	TOTAL:	5.00	EACH
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Total Beam Lines = 5



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

74567

PHASE

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ORG

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SHEET #: 30 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

518E21200	POROUS BACKFILL WITH GEOTEXTILE FABRIC	TOTAL:	89.00	CY
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Abutments

Total Top area = 148.49 sf

Top area of Wingwall Blocks = 31.989 sf

Top Area of Wingwall Fillet = 2.335 sf

Top Area of Corner Fillet = 2.335 sf

Height of Wingwall Block = 4.90 ft

Height of Wingwall Fillet = 4.90 ft

Height of Corner Fillet = 2.57 ft

Volume of Concrete in Way = 174.31 cf

Volume of Total Top Area = 1368.9 cf

Volume Per Abutment = 1194.6 cf

= 44.246 cy

Total = 89 cy



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

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SHEET #: 31 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

518E40000	6" PERFORATED CORRUGATED PLASTIC PIPE	TOTAL:	114.00	FT
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Total = 114 ft See LAK-6 Drainage Pipe Length Calculation



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

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SHEET #: 32 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

518E40010	6" NON-PERFORATED CORRUGATED PLASTIC PIPE, INCLUDING SPECIALS	TOTAL:	102.00	FT
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Grand Total = 102 ft See Drainage Pipe Length Calculation



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

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SHEET #: 33 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

526E25001	REINFORCED CONCRETE APPROACH SLABS (T=15"), AS PER PLAN	TOTAL:	209.00	SY
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Reinforced Bridge Approach Slab

Area Measured in CAD for Rear Approach = 99.39 sy

Area Measured in CAD for Forward Approach = 109.08 sy



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

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SHEET #: 34 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

526E90010	TYPE A INSTALLATION	TOTAL:	83.00	FT
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Rear Approach = 40.433 ft

Forward Approach = 41.71 ft * slightly different than usual, but similar



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 35 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

601E20000	CRUSHED AGGREGATE SLOPE PROTECTION	TOTAL:	642.00	SY
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Rear Abutment

Slope = 26.565 deg

Area measured in CAD = 2350.6 sf Area adjusting from Slope = 1.118

Toe of Slope Key Length = 75.621 ft

Area = 2703.7 sf

= 300.41 sy

Forward Abutment

Area measured in CAD = 2674.2 sf Area adjusting from Slope = 1.118

Toe of Slope Key Length = 78.477 ft

Area = 3068.4 sf

= 340.93 sy



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 36 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

607E39901	VANDAL PROTECTION FENCE, 6' STRAIGHT, COATED FABRIC, AS PER PLAN	TOTAL:	742.00	FT
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Length of Protection fence over North Railing = 380 ft

Length of Protection fence over South Railing = 361.635 ft

Total = 742 ft



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

74567

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SHEET #: 37 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

625E33001	STRUCTURE GROUNDING SYSTEM, AS PER PLAN	TOTAL:	1.00	EACH
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Abutment Electrodes = 4 (Note 4 in HL-50.21)

Number at Pier 2 = 2 (Note 5 in HL-50.21)

Number at Pier 4 = 2 (Note 3 in HL-50.21)

Total = 8



CHA COMPUTATION PAD

COMPLETED BY: CAE

CHECKED BY: DOR

PROJECT NAME: LAK-6-2.06

PROJECT LOCATION: Willoughby Hills (LAK County)

PROJECT

PHASE

ORG

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SHEET #: 38 OF 38

DATE: 3/4/2025

SUBJECT: Quantity Calculations

846E00111	POLYMER MODIFIED ASPHALT EXPANSION JOINT SYSTEM, AS PER PLAN	TOTAL:	35.00	CF
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Width = 1.6667 ft

Thickness = 0.25 ft

Length of Joint Rear = 40.433 ft

Length of Joint Fwd. = 41.71 ft

Total = 35 cf