MAR-529-2.59 ODOT PID NO: 112818 MARION COUNTY, OHIO

DRAFT SUBGRADE EXPLORATION REPORT

Prepared For: ODOT District Six 400 East William Street Delaware, OH 43015

Prepared By: Resource International, Inc. 6350 Presidential Gateway Columbus, OH 43231

Rii Project No. W-20-161

April 2021

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April 21, 2021

Mr. Anthony Turowski, P.E. Planning Engineer ODOT District Six 400 East William Street Delaware, Ohio 43015

Re: Draft Subgrade Exploration Report MAR-529-2.59 ODOT PID No. 112818 Marion County, Ohio Rii Project No. W-20-161

Mr. Turowski:

Resource International, Inc. (Rii) is pleased to submit this draft subgrade exploration report for the above referenced project. Engineering logs have been prepared and are attached to this report along with the results of laboratory testing. This report includes recommendations for the proposed improvements along SR 529 between SLM 2.59 and 9.33 in Marion County, Ohio.

We sincerely appreciate the opportunity to be of service to you on this project. If you have any questions regarding the Subgrade exploration or this report, please contact us.

Sincerely,

Enclosure:

RESOURCE, INTERNATIONAL, INC.

Peyman P. Majidi, P.E. Project Engineer

Jonathan P. Sterenberg, P.E. Vice President – Geotechnical Services

Planning

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Construction Management

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Draft Subgrade Exploration Report

Sectio	on Page	e
EXEC	UTIVE SUMMARY	I
	Exploration and Findings Analysis and Recommendations	i i
1.0		1
2.0	GEOLOGY AND OBSERVATIONS OF THE PROJECT	1
	 2.1 Site Geology	1 2
3.0	EXPLORATION	2
4.0	FINDINGS	4
	 4.1 Surface Materials	4 5 5 5
5.0	ANALYSES AND RECOMMENDATIONS	6
	 5.1 Pavement Subgrade Recommendations	6 6 7 8 8 9 9
	5.4 Groundwater Considerations	9
6.0	LIMITATIONS OF STUDY	0

TABLE OF CONTENTS

APPENDICES

Appendix I	Vicinity Map and Boring Plan
Appendix II	Description of Soil Terms
Appendix III	Boring Logs: B-001-0-20 through B-089-0-20
Appendix IV	MAR-529-2.59 DCP logs
Appendix V	Photographic Pavement Core Logs
Appendix VI	GB1 Subgrade Stabilization Summary

EXECUTIVE SUMMARY

Resource International, Inc. (Rii) has completed a subgrade exploration report for the design and construction of the proposed improvements to the eastbound and westbound shoulders of State Route 529 between SLM 2.59 and 9.33 in Marion County. It is understood that consideration is being given to full depth reclamation (FDR) for the shoulder reconstruction.

Exploration and Findings

Between March 1 and 9, 2021, a total of forty-five (45) borings, designated as B-001-0-20 through B-089-0-20 were drilled by ODOT drilling crews to completion depths of 7.5 feet below the existing pavement surface. In addition, forty-four (44) dynamic cone penetrometer (DCP), designated as D-003-021 through D-088-0-21 were performed to depth of between 30 to 40 inches below the bottom of the existing pavement. The boring and DCP locations are illustrated on the boring plan presented in Appendix I of this report.

All borings and DCP tests were performed within the existing pavement. Pavement cores were obtained from selected boring locations. The borings and pavement cores encountered 8.0 to 14.0 inches of reinforced concrete pavement underlain by 2.0 to 10.5 inches of aggregate base. Pavement thicknesses in the DCP tests were measured between 4.0 and 16.0 inches of asphalt.

Underlying the pavement materials in the borings, predominantly natural cohesive soils with seams of granular soils were encountered to boring termination depths.

Analysis and Recommendations

Pavement Subgrade Recommendations

The subgrade soils along the alignment are anticipated to consist of predominantly cohesive materials comprised of soft to hard clay, silty clay, silt and clay, sandy silt and silt. Based on the soil conditions encountered during the drilling phase, it is estimated that the subgrade soils within the upper portions of the proposed subgrade will require some level of stabilization under ODOT GB1. Based on the results of the GB1 analysis, the overall average site parameters based on all of the soil borings performed as part of this exploration are as follows:

Average	Average	Average	Average Optimum	Average	Design
N _{60L}	Pl	Moisture	Moisture	Group Index	CBR
10	19	19	16	12	5

Overall Average Site Parameters



Applying the averages in the preceding table and based on the results of the GB-1 analysis, the following global stabilization options are recommended within the project limits:

- Option 1. Chemically stabilize the entire subgrade with 14-inches of cement, as per ODOT Construction and Materials Specification (CMS) Item 206. For estimating purposes, utilize a cement content of 5.0 percent by weight of soil. Actual application rates shall be verified by the contractor under Item 206.06 Mixture Design for Chemically Stabilized Soils.
- Option 2. Stabilize the entire subgrade via a 12-inch undercut and replacement with ODOT item 703.16C granular material, Type B, C or D installed over ODOT Item 712.09 Geotextile Fabric, Type D as detailed in accordance with ODOT Item 204.

Per ODOT GB1 requirements, the entire subgrade should be stabilized using one of the global stabilization options provided above. Upon completion of the stabilization, the entire subgrade should be proof rolled to verify that stability has been achieved.

California Bearing Ratio (CBR) values from the soil borings had an average of 5, while DCP values displayed average CBR values ranging from 4 to 12 with average site CBR value of 7. Based on the conditions encountered across the subject site, **it is recommended that pavement design be based on a CBR value of 5** with a corresponding resilient modulus, MR, of 8,400 psi. Correlation charts indicate a modulus of subgrade reaction (K) of 135 pci and a soil support value (SSV) of 3.8.

Unsuitable subgrade soil consisting of silt (ODOT A-4b) soil was identified in boring B-018-0-20 and B-086-0-20 to depth of 3.0 feet below existing grade. If excavation and replacement option has been elected as the preferred alternative, silt encountered within the subgrade should be over excavated to a depth of 36 inches below the top of the proposed subgrade or to its entirety, whichever is less. For planning purposes, it is assumed the unsuitable subgrade soils (ODOT A-4b) extends from mid length between borings B-017-0-20 and B-021-0-20 and between borings B-085-0-20 and B-089-0-20, for an overall length of approximately 2,400 feet.

It is understood that ODOT is considering full depth reclamation (FDR) for the reconstruction of the shoulder pavement. Rii recommends the use of FDR with cement as a stabilizing agent.

Mix designs for the FDR should be performed in accordance with ODOT Special Provision for FDR Chemical Stabilization, dated March 24, 2010. Rii can be available to provide a mix design for the full depth reclamation using samples of the existing materials and the stabilizing agent (cement). For preliminary estimating purposes, a cement content of 6



percent may be considered, along with a total treatment depth of 12 inches from the top of the pavement surface.

Please note that this executive summary does not contain all the information presented in the report. The unabridged Subgrade exploration report should be read in its entirety to obtain a more complete understanding of the information presented.



1.0 INTRODUCTION

This report is a presentation of the subgrade exploration performed for the design and construction of the proposed improvements to the eastbound and westbound shoulders of State Route 529 between SLM 2.59 and 9.33 in Marion County. It is understood that consideration is being given to full depth reclamation (FDR) for the shoulder reconstruction. The project area is shown on the vicinity map presented in Appendix I.

2.0 GEOLOGY AND OBSERVATIONS OF THE PROJECT

2.1 Site Geology

Both the Illinoian and Wisconsinan glaciers advanced over two-thirds of the State of Ohio, leaving behind glacial features such as moraines, kame deposits, lacustrine deposits and outwash terraces. The glacial and non-glacial regions comprise five physiographic sections grouped by age, depositional process and geomorphic occurrence. Physiographically, the site lies within the Central Ohio Clayey Till Plain Region of the Till Plains Section. The Central Ohio Clayey Till Plains is covered with well-defined ground and end moraines composed of clayey till deposits with sparse lake basins throughout. A ground moraine is the sheet of debris left after a steady retreat of the ice. And end moraine is a ridge like accumulation of till marking a standstill position of a present or past glacial front. The debris left behind by the ice ranges in composition, from clay sizes to boulders (including silt, sand, and gravel). A lake basin contains lacustrine sand in laminated to thin sheets.

Based on the bedrock geology and topography maps of the site, obtained from the Ohio Department of Natural Resources (ODNR), the bedrock underlying the project is comprised of three different units ranging from Middle Devonian to Upper Devonian in age. The project traverses, from west to east and in ascending order of age, the Delaware Limestone, the Olentangy Shale, and the Ohio Shale Formations. The Delaware Limestone lies beneath the western portion of the project from west of US 23 to just west of SR-98. The Olentangy Shale underlies the project from just west of SR 98 to approximately 0.4 miles east of SR 98. The rest of the project, from 0.4 miles east of SR 98 to the eastern project limit, is underlain by the Ohio Shale Formation. The contacts between these formations are separated by unconformities or breaks or gaps in the geologic record either by an interruption in deposition or an erosion event. The Delaware Limestone can be characterized as being argillaceous, cherty, and carbonaceous, gray to brown in color, with thin to massive bedding planes. The limestone ranges in thickness from 0 to 105 feet thick with chert nodules and chert layers, along with a petroliferous odor. The Olentangy Shale formation consists of greenish gray (upper portion) to gray (lower portion), clayey, pyritic shale which locally contains lenses and nodules of limestone. This formation ranges from 20 to 55 feet thick. The upper most unit encountered beneath the site is the Ohio Shale Formation. The Ohio Shale Formation is further subdivided into three (3) members (in descending order), the Cleveland, Chagrin, and Huron Members. The Cleveland Member consists of black shale which is thickest in

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1



the north-central portion of Ohio, but thins towards the south and east and is absent in Northeastern Ohio. The Chagrin Member consists of gray to greenish-gray shale, siltstone, and very fine-grained sandstone, eventually grading into either the overlying Cleveland Member or the underlying Huron Member. The Huron Member is mostly comprised of black, carbonaceous shale with calcareous concretions common in the lower portion of the unit. The Ohio Shale Formation ranges between approximately 250 and 500 feet thick.

The bedrock surface is somewhat hummocky and irregular in the vicinity of the site, but generally slopes upward from west to east beneath the project limits. The bedrock is lowest in a small valley, which runs north-south between US 23 and SR 98, at an elevation of approximately 920 feet mean sea level (msl). The bedrock surface rises slightly between SR 98 and Mautz Yeager Road, then slopes downward slightly from there to East River Road, and then rises again to a crest beneath Claridon Ashley Road where it is highest at an elevation of 960+ feet msl. None of the borings drilled for this project encountered the bedrock surface. With surface elevations at the boring locations ranging from approximately 971 feet to 1005 feet, the depth to bedrock lies approximately 35 to 60 feet below the ground surface.

2.2 Existing Site Conditions

The project site is located along the alignment of Marion-Cardington Road East, SR 529 in the southeast part of city of Marion, the county seat of Marion County, Ohio. SR 529 is classified as a major collector within the limits of the project, and is situated in an east-west direction. The roadway accommodates one lane of traffic in each direction. The roadway predominantly spans farm fields.

3.0 EXPLORATION

Between March 1 and 9, 2021, a total of forty-five (45) borings, designated as B-001-0-20 through B-089-0-20 were drilled by ODOT drilling crews to completion depths of 7.5 feet below the existing pavement surface. In addition, forty-four (44) dynamic cone penetrometer (DCP), designated as D-003-021 through D-088-0-21 were performed to depth of between 30 to 40 inches below the bottom of the existing pavement. The boring and DCP locations are illustrated on the boring plan presented in Appendix I of this report.

GPS coordinates of the boring and DCP locations were obtained by RII using a handheld GPS unit. Ground surface elevations were estimated using Google Earth.

The borings were drilled with a CME-55 truck mounted rotary drilling machine, utilizing a 3.25-inch inside diameter, hollow stem auger to advance the holes. In general, standard penetration test (SPT) and split spoon sampling were performed using continuous sampling to the boring termination depth within each of the borings. The SPT, per the American Society for Testing and Materials (ASTM) designation D1586, is conducted



using a 140-pound hammer falling 30.0 inches to drive a 2.0-inch outside diameter split spoon sampler 18.0 inches. Rii utilized a calibrated automatic drop hammer to generate consistent energy transfer to the sampler. Driving resistance is recorded on the boring logs in terms of blows per 6.0-inch interval of the driving distance. The second and third intervals are added to obtain the number of blows per foot (N). Standard penetration blow counts aid in determining soil properties applicable in pavement and foundation system design. Measured blow count (N) values are corrected to an equivalent (60%) energy ratio, N₆₀, by the following equation. Both values are represented on boring logs in Appendix III.

$$N_{60} = N_m^*(ER/60)$$

Where: N_m = measured N value ER = drill rod energy ratio, expressed as a percent, for the system used

The automatic hammer for the CME-55 drill rig used for this project was calibrated on September 15, 2020 and has a drill rod energy ratio of 83.6 percent. Upon completion of drilling, the borings were backfilled with either soil cuttings generated during the drilling process or a mixture of soil cuttings and bentonite hole plug. Where borings penetrated the existing subgrade, the pavement surface was patched with an equivalent thickness of quickset concrete. During drilling, Rii personnel prepared field logs showing the encountered subsurface conditions. Soil samples obtained from the drilling operation were preserved and sealed in glass jars and delivered to the soil laboratory. In the laboratory, the soil samples were visually classified and select samples were tested, as noted in Table 1.

Laboratory Test	Test Designation	Number of Tests Performed
Natural Moisture Content	ASTM D 2216	184
Plastic and Liquid Limits	AASHTO T89, T90	93
Gradation – Sieve/Hydrometer	AASHTO T88	94
Sulfate Content – Colorimetric Method	ODOT \$1122	46

 Table 1. Laboratory Test Schedule

The tests performed are necessary to classify existing soil according to the Ohio Department of Transportation (ODOT) classification system and to estimate engineering properties of importance for pavement and foundation design and construction recommendations. Results of the laboratory testing are presented on the boring logs in Appendix III. A description of the soil terms used throughout this report is presented in Appendix II.



Hand penetrometer readings, which provide a rough estimate of the unconfined compressive strength of the soil, were reported on the boring logs in units of tons per square foot (tsf) and were utilized to classify the consistency of the cohesive soil in each layer. An indirect estimate of the unconfined compressive strength of the cohesive split spoon samples can also be made from a correlation with the blow counts (N₆₀). Please note that split spoon samples are considered to be disturbed and the laboratory determination of their shear strengths may vary from undisturbed conditions.

4.0 FINDINGS

Interpreted engineering logs have been prepared based on the field logs, visual examination of samples and laboratory test results. Classification follows the current version of the ODOT Specifications for Subgrade Explorations (SGE). The following is a summary of what was found in the test borings and what is represented on the boring logs.

4.1 Surface Materials

Pavement cores were obtained from selected boring and DCP locations. Table 2 displays the pavement thickness and aggregate base in each boring.

Boring ID	Asphalt Thickness (in)	Aggregate Base Thickness (in)
B-002-0-20	12.25	4.0
B-017-0-20	8.0	10.5
B-022-0-20	10.0	8.0
B-037-0-20	10.0	8.0
B-042-0-20	11.25	6.0
B-057-0-20	2.75	6.0
B-062-0-20	11.5	5.0
B-077-0-20	10.5	8.0
B-082-0-20	10.0	4.0

Table 2. Summary of Pavement Cores

It is understood that that pavement cores were performed within the existing pavement. The borings and pavement cores encountered between 8.0 to 14.0 inches of asphalt overlaying 2.0 to 10.5 inches of aggregate base. The cores were inspected in lab and appeared to be in highly degraded to fair condition. For further details and photographic logs of the pavement cores please see Appendix IV. Pavement thicknesses in the DCP tests were measured between 4.0 and 16.0 inches of asphalt.

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4



4.2 Subsurface Soils

Underlying the pavement materials in the borings, predominantly natural cohesive soils with seams of granular soils were encountered to boring termination depths. The cohesive soils were described as brown, brownish gray, dark gray clay, silt and clay, silty clay, sandy silt and silt (ODOT A-7-6, A-6b, A-6a, A-4a and A-4b). Seams of granular soils were generally described as brown sand and gravel, gravel with sand and silt and gravel with sand, silt and clay (ODOT A-1-b, A-2-4 and A-2-6).

The shear strength and consistency of the cohesive soils are primarily derived from the hand penetrometer values (HP). The cohesive soils encountered across the site ranged from soft ($0.25 < HP \le 0.5$ tsf) to hard (4.5 < HP tsf). The unconfined compressive strength of the cohesive soil samples tested, obtained from the hand penetrometer, ranged from 0.5 tsf to over 4.5 tsf (limit of instrument).

Natural moisture contents of the soil samples tested ranged from 6 to 30 percent. In general, the soils exhibited natural moisture contents estimated to be slightly above optimum moisture levels.

Sulfate testing was performed in all of the borings in accordance with the ODOT S1122 Colorimetric Method in the upper soils of the existing subgrade along the proposed alignments, as outlined in the current ODOT SGE and Geotechnical Bulletin Number 1: Plan Subgrades (GB1). Based on the results of the testing, the sulfate contents of the subgrade soils range from 47 to 390 parts per million (ppm or mg/kg of material). Results of the sulfate testing at each boring location tested are provided on the respective boring log in Appendix III.

4.3 Bedrock

Bedrock was not encountered in any of the borings performed for this investigation.

4.4 Groundwater

Groundwater was initially encountered during in boring B-026-0-20 and B-029-0-20 at the depth of 6.0 and 4.0 feet below existing grade, respectively. Groundwater was not encountered in any remaining borings, either during or at completion of drilling.

Please note that short-term water level readings, especially in cohesive soils, are not necessarily an accurate indication of the actual groundwater level. In addition, groundwater levels or the presence of groundwater are considered to be dependent on seasonal fluctuations in precipitation. A more comprehensive description of what was encountered during the drilling process may be found on the boring logs in Appendix III.



5.0 ANALYSES AND RECOMMENDATIONS

Data obtained from the drilling and testing program have been used to determine pavement foundation and support capabilities for the soils encountered at the site. These parameters have been used to provide guidelines for the design of the pavement foundation systems, as well as the construction specifications related to the placement of the pavement and general earthwork recommendations, which are discussed in the following paragraphs.

DCP data provided by ODOT is utilized in confirming and corroborating the CBR values determined from subgrade soil parameter.

5.1 Pavement Subgrade Recommendations

The subgrade soils along the alignment, within the project corridor, are anticipated to consist of predominantly cohesive materials comprised of soft to hard clay, silty clay, silt and clay, sandy silt and silt (ODOT A-7-6, A-6b, A-6a, A-4a and A-4b). Based on the soil conditions encountered during the drilling phase, it is estimated that the subgrade soils within the upper portions of the proposed subgrade will require some level of stabilization under ODOT GB1. Profile information was not available at the time of this report; however, it is anticipated that the proposed subgrade will generally match the existing subgrade, and that minor amounts of earthwork cut or fill may be required to achieve the proposed subgrade elevations.

5.1.1 Subgrade Stabilization

Based on the ODOT GB1 guidelines, when approximately 30 percent or more of the subgrade area requires stabilization, consideration should be given to utilizing a global stabilization option. For this project, approximately 77 percent of the subgrade area is anticipated to require stabilization based on the soil borings performed. Per ODOT GB1, global stabilization recommendations are based upon the overall average site parameters, as noted in Table 3.

Average N₀₀∟	Average Pl	Average Moisture	Average Optimum Moisture	Average Group Index	Average CBR
10	19	19	16	12	5

Table 3. Average Site Parameters

Applying the averages in Table 3 and based on the results of the GB-1 analysis the following global stabilization options are recommended within the project limits:

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6



- Option 3. Chemically stabilize the entire subgrade with 14-inches of cement, as per ODOT Construction and Materials Specification (CMS) Item 206. For estimating purposes, utilize a cement content of 5.0 percent by weight of soil. Actual application rates shall be verified by the contractor under Item 206.06 Mixture Design for Chemically Stabilized Soils.
- Option 4. Stabilize the entire subgrade via a 12-inch undercut and replacement with ODOT item 703.16C granular material, Type B, C or D installed over ODOT Item 712.09 Geotextile Fabric, Type D as detailed in accordance with ODOT Item 204.

Per ODOT GB1 requirements the entire subgrade should be stabilized using one of the global stabilization options provided above. Upon completion of the stabilization, the entire subgrade should be proof rolled to verify that stability has been achieved.

5.1.2 Subgrade Design Considerations

California Bearing Ratio (CBR) values from the soil parameters from the soil borings ranged from 3 to 12 with an average of 5, while DCP values displayed average CBR values ranging from 4 to 12 with average site CBR value of 7. Based on the conditions encountered across the subject site, **it is recommended that pavement design be based on a CBR value of 5** with a corresponding resilient modulus, M_R , of 6,000 psi. Correlation charts indicate a modulus of subgrade reaction (K) of 135 pci and a soil support value (SSV) of 3.8.

Per ODOT GB1, soils with sulfate content in excess of 5,000 ppm cannot be chemically stabilized due to the potential for sulfate heave in the soil. Based on the results of the testing, the sulfate contents of the subgrade soils range from 47 to 390 ppm. Therefore, soil with sulfate content greater than 5,000 ppm was not encountered in any boring.

Please note that the recommended CBR values assume that the materials utilized for the subgrade in fill areas are equivalent to, or better than materials at the existing subgrade elevation. Sources of borrow material should be designated in advance of construction. The material should be tested in the laboratory to verify the soil exhibits a minimum design CBR value of 5.

Pavement design is dependent on the inclusion of adequate surface and subsurface drainage in order to maintain the compacted subgrade near optimum moisture conditions throughout the lifetime of the pavement. If underdrain systems are considered, they should be installed in accordance to the specifications presented in Item 204 of the ODOT CMS.



5.1.3 Unsuitable Subgrade Recommendations

Unsuitable subgrade soil consisting of silt (ODOT A-4b) soil was identified in boring B-018-0-20 and B-086-0-20 to depth of 3.0 feet below existing grade. If excavation and replacement option has been elected as the preferred alternative, silt encountered within the subgrade should be over excavated to a depth of 36 inches below the top of the proposed subgrade or to its entirety, whichever is less. For planning purposes, it is assumed the unsuitable subgrade soils (ODOT A-4b) extends from mid length between borings B-017-0-20 and B-021-0-20 and between borings B-085-0-20 and B-089-0-20, for an overall length of approximately 2,400 feet. Over excavations should be replaced with ODOT Item 204 Embankment. If cement stabilization option has been elected as the preferred alternative, then 14-inches of stabilization depth should be considered. Actual limits of the unsuitable subgrade and corresponding undercut should be determined by the Project Engineer in the field based on the results of proof rolling and subgrade observations in accordance with ODOT CMS Item 204 as well as guidance provided under Item 204 in the ODOT Construction Administration Manual of Procedures (MOP).

5.2 Full Depth Reclamation

It is understood that ODOT is considering full depth reclamation (FDR) for the reconstruction of the shoulder pavement. Rii recommends the use of FDR with cement as a stabilizing agent. This method is a process where the existing wearing asphalt is milled, while the remaining material including base, subbase as well as portions of the subgrade section is pulverized and mixed with sufficient water and stabilizing agent (i.e. cement). This mix is then used as an asphalt treated base course. After the pavement has had time to cure, a wearing surface is then applied to the reclaimed surface. This is primarily used to protect the recycled pavement from water entering into the mix and causing distress or early failures.

Among the benefits of this option are: increased structural capacity and durability, as well as a shortened construction schedule. There would be minimal need to haul off the millings, as the majority of these would be incorporated into the mix of the stabilized base course.

Mix designs for the FDR should be performed in accordance with ODOT Special Provision for FDR Chemical Stabilization, dated March 24, 2010. Rii can be available to provide a mix design for the full depth reclamation using samples of the existing materials and the stabilizing agent (cement). For preliminary estimating purposes, a cement content of 6 percent may be considered, along with a total treatment depth of 12 inches from the top of the pavement surface.



5.3 Construction Considerations

All site work shall conform to local codes and to the latest ODOT CMS, including that all excavation and embankment preparation and construction should follow ODOT Item 200 (Earthwork).

5.3.1 Excavation Considerations

All excavations should be shored / braced or laid back at a safe angle in accordance to Occupational Safety and Health Administration (OSHA) guidelines. During excavation, if slopes cannot be laid back to OSHA Standards due to adjacent structures or other obstructions, temporary shoring may be required. The following table should be utilized as a general guide for implementing OSHA guidelines when estimating excavation back slopes at the various boring locations. Actual excavation back slopes must be field verified by qualified personnel at the time of excavation in strict accordance with OSHA guidelines.

Soil	Maximum Back Slope	Notes
Soft to Medium Stiff Cohesive	1.5 : 1.0	Above Ground Water Table and No Seepage
Stiff Cohesive	1.0 : 1.0	Above Ground Water Table and No Seepage
Very Stiff to Hard Cohesive	0.75 : 1.0	Above Ground Water Table and No Seepage
All Granular & Cohesive Soil Below Ground Water Table or with Seepage	1.5 : 1.0	None

Table 4. Excavation Back Slopes

5.4 Groundwater Considerations

Based on the groundwater observations made during drilling, seepage and/or groundwater is not anticipated to be encountered during construction at the site. Where/if groundwater is encountered, proper groundwater control measures should be implemented to prevent disturbance to excavation bottoms consisting of cohesive soil, and to prevent the possible development of a quick or "boiling" condition if soft/loose silts and/or fine sands are encountered. It is preferable that the groundwater level, if encountered, be maintained at least 24.0 inches below the deepest excavation. Any seepage or groundwater encountered at this site should be able to be controlled by pumping from temporary sumps. Note that determining and maintaining actual groundwater levels during construction is the responsibility of the contractor.



6.0 LIMITATIONS OF STUDY

The above recommendations are predicated upon construction inspection by a qualified soil technician under the direct supervision of a professional Subgrade engineer. Adequate testing and inspection during construction are considered necessary to assure an adequate foundation system and are part of these recommendations.

The recommendations for this project were developed utilizing soil and bedrock information obtained from the test borings that were made at the proposed site for the current investigation. Resource International is not responsible for the data, conclusions, opinions or recommendations made by others during previous investigations at this site. At this time we would like to point out that soil borings only depict the soil and bedrock conditions at the specific locations and time at which they were made. The conditions at other locations on the site may differ from those occurring at the boring locations.

The conclusions and recommendations herein have been based upon the available soil and bedrock information and the design details furnished by a representative of the owner of the proposed project. Any revision in the plans for the proposed construction from those anticipated in this report should be brought to the attention of the Subgrade engineer to determine whether any changes in the foundation or earthwork recommendations are necessary. If deviations from the noted subsurface conditions are encountered during construction, they should also be brought to the attention of the Subgrade engineer.

The scope of our services does not include any environmental assessment or investigation for the presence or absence of hazardous or toxic materials in the soil, groundwater or surface water within or beyond the site studied. Any statements in this report or on the test boring logs regarding odors, staining of soils or other unusual conditions observed are strictly for the information of our client.

Our professional services have been performed, our findings obtained and our recommendations prepared in accordance with generally accepted Subgrade engineering principles and practices. Resource International is not responsible for the conclusions, opinions or recommendations made by others based upon the data included.

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10



APPENDIX I

VICINITY MAP AND BORING PLAN



ODOT District 6 GES Task 6-M MAR-529-2.59 Boring Plan Rii Project No. W-20-161 April 9, 2021





ODOT District 6 GES Task 6-M MAR-529-2.59 Boring Plan Rii Project No. W-20-161 April 9, 2021





ODOT District 6 GES Task 6-M MAR-529-2.59 Boring Plan Rii Project No. W-20-161 April 9, 2021



APPENDIX II

DESCRIPTION OF SOIL TERMS

DESCRIPTION OF SOIL TERMS

The following terminology was used to describe soils throughout this report and is generally adapted from ASTM 2487/2488 and ODOT Specifications for Geotechnical Explorations.

<u>Granular Soils</u> - The relative compactness of granular soils is described as: ODOT A-1, A-2, A-3, A-4 (non-plastic) or USCS GW, GP, GM, GC, SW, SP, SM, SC, ML (non-plastic)

Description	Blows per	foot –	SPT (N ₆₀)
Very Loose	Below		5
Loose	5	-	10
Medium Dense	11	-	30
Dense	31	-	50
Very Dense	Over		50

<u>Cohesive Soils</u> - The relative consistency of cohesive soils is described as: ODOT A-4, A-5, A-6, A-7, A-8 or USCS ML, CL, OL, MH, CH, OH, PT

	Unconfined		ed
Description	Compression (tsf)		<u>n (tsf)</u>
Very Soft	Less than		0.25
Soft	0.25	-	0.5
Medium Stiff	0.5	-	1.0
Stiff	1.0	-	2.0
Very Stiff	2.0	-	4.0
Hard	Over		4.0

Gradation - The following size-related denominations are used to describe soils:

Soil Fraction		USCS Size
Boulders		Larger than 12"
Cobbles		12" to 3"
Gravel	coarse	3" to ¾"
	fine	3⁄4" to 4.75 mm (3⁄4" to #4 Sieve)
Sand	coarse	4.75 mm to 2.0 mm (#4 to #10 Sieve)
	medium	2.0 mm to 0.42 mm (#10 to #40 Sieve)
	fine	0.42 mm to 0.074 mm (#40 to #200 Sieve)
Silt		0.074 mm to 0.005 mm (#200 to 0.005 mm)
Clay		Smaller than 0.005 mm

Modifiers of Components - Modifiers of components are as follows:

Term		Range	
Trace	0%	-	10%
Little	10%	-	20%
Some	20%	-	35%
And	35%	-	50%

Moisture Table - The following moisture-related denominations are used to describe cohesive soils:

<u>Term</u>	Range - USCS	Range - ODOT
Dry	0% to 10%	Well below Plastic Limit
Damp	>2% below Plastic Limit	Below Plastic Limit
Moist	2% below to 2% above Plastic Limit	Above PL to 3% below LL
Very Moist	>2% above Plastic Limit	
Wet	≥ Liquid Limit	3% below LL to above LL

Organic Content – The following terms are used to describe organic soils:

Term	Organic Content (%)
Slightly organic	2-4
Moderately organic	4-10
Highly organic	>10

<u>Bedrock</u> – The following terms are used to describe the relative strength of bedrock:

Description	Field Parameter
Very Weak	Can be carved with knife and scratched by fingernail. Pieces 1 in. thick can be broken by finger pressure.
Weak	Can be grooved or gouged with knife readily. Small, thin pieces can be broken by finger pressure.
Slightly Strong	Can be grooved or gouged 0.05 in deep with knife. 1 in. size pieces from hard blows of geologist hammer.
Moderately Strong	Can be scratched with knife or pick. 1/4 in. size grooves or gouges from blows of geologist hammer.
Strong	Can be scratched with knife or pick with difficulty. Hard hammer blows to detach hand specimen.
Very Strong	Cannot be scratched by knife or pick. Hard repeated blows of geologist hammer to detach hand specimen.
Extremely Strong	Cannot be scratched by knife or pick. Hard repeated blows of geologist hammer to chip hand specimen.

ODOT Size Larger than 12" 12" to 3" 3" to 3/4" 3/4" to 2.0 mm (3/4" to #10 Sieve) 2.0 mm to 0.42 mm (#10 to #40 Sieve)

0.42 mm to 0.074 mm (#40 to #200 Sieve) 0.074 mm to 0.005 mm (#200 to 0.005 mm) Smaller than 0.005 mm



CLASSIFICATION OF SOILS Ohio Department of Transportation

(The classification of a soil is found by proceeding from top to bottom of the chart. The first classification that the test data fits is the correct classification.)

SYMBOL	DESCRIPTION	Classifo AASHTO	ation OHIO	LL _O /LL × 100*	% Pass #40	% Pass #200	Liquid Limit (LL)	Plastic Index (PI)	Group Index Max.	REMARKS
	Gravel and/or Stone Fragments	Α-	1-a		30 Max.	15 Max.		6 Max.	0	Min. of 50% combined gravel, cobble and boulder sizes
	Gravel and/or Stone Fragments with Sand	A - 1	1-Ь		50 Max.	25 Max.		6 Max.	0	
F S	Fine Sand	A	- 3		51 Min.	10 Max.	NON-PI	_ASTIC	0	
	Coarse and Fine Sand		A-3a			35 Max.		6 Max.	0	Min. of 50% combined coarse and fine sand sizes
0.0.0 0.0.0 0.0.0 0.0.0 0.0.0	Gravel and/or Stone Fragments with Sand and Silt	A	2-4 2-5			35 Max.	40 Max. 41 Min.	10 Max.	0	
0.0.0 0.00 0.00 0.00 0.00 0.00 0.00 0.	Gravel and/or Stone Fragments with Sand, Silt and Clay	A-:	2-6 2-7			35 Max.	40 Max. 41 Min.	11 Min.	4	
	Sandy Sil†	A-4	A-4a	76 Min.		36 Min.	40 Max.	10 Max.	8	Less †han 50% sil† sizes
+ + + + + + + + + + + + + + + + + + +	silt	A-4	A-4b	76 Min.		50 Min.	40 Max.	10 Max.	8	50% or more silt sizes
	Elastic Silt and Clay	A	-5	76 Min.		36 Min.	41 Min.	10 Max.	12	
	Silt and Clay	A-6	A-6a	76 Min.		36 Min.	40 Max.	11 - 15	10	
	Silty Clay	A-6	A-6b	76 Min.		36 Min.	40 Max.	16 Min.	16	
	Elastic Clay	Α-	7-5	76 Min.		36 Min.	41 Min.	≦LL-30	20	
	Clay	Α-	7-6	76 Min.		36 Min.	41 Min.	>LL-30	20	
+ + + + + + + +	Organic Silt	A-8	A-8a	75 Max.		36 Min.				W∕o organics would classify as A-4a or A-4b
	Organic Clay	A-8	A-8b	75 Max.		36 Min.				W/o organics would classify as A-5, A-6a, A-6b, A-7-5 or A-7-6
	MAT	ERIAL	CLASS	SIFIED B	Y VISUAL	INSPEC	FION			
	Sod and Topsoil $\wedge \rightarrow > V$ Pavement or Base $\sim \wedge \land \land$ $\downarrow \rightarrow \downarrow$ $\downarrow \rightarrow \downarrow$	Uncon Fill (E	trolled escribe)		Bouldery	/ Zone		PPe	o†

* Only perform the oven-dried liquid limit test and this calculation if organic material is present in the sample.

APPENDIX III

BORING LOGS:

B-001-0-20 through B-089-0-20

BORING LOGS

Definitions of Abbreviations

- AS = Auger sample
- HP = Unconfined compressive strength as determined by a hand penetrometer (tons per square foot)
- LOI = Percent organic content (by weight) as determined by ASTM D2974 (loss on ignition test)
- PID = Photo-ionization detector reading (parts per million)
- QR = Unconfined compressive strength of intact rock core sample as determined by ASTM D2938 (pounds per square inch)
- QU = Unconfined compressive strength of soil sample as determined by ASTM D2166 (pounds per square foot)
- RC = Rock core sample
- REC = Ratio of total length of recovered soil or rock to the total sample length, expressed as a percentage
- RQD = Rock quality designation estimate of the degree of jointing or fracture in a rock mass, expressed as a percentage:

$\frac{\sum \text{ segments equal to or longer than 4.0 inches}}{\text{core run length}} x 100$

- S = Sulfate content (parts per million)
- SPT = Standard penetration test blow counts, per ASTM D1586. Driving resistance recorded in terms of blows per 6-inch interval while letting a 140-pound hammer free fall 30 inches to drive a 2-inch outer diameter (O.D.) split spoon sampler a total of 18 inches. The second and third intervals are added to obtain the number of blows per foot (N).
- SS = Split spoon sample
- For instances of no recovery from standard SS interval, a 2.5 inch O.D. split spoon is driven the full length of the standard SS interval plus an additional 6.0 inches to obtain a representative sample. Only the final 6.0 inches of sample is retained. Blow counts from 2S sampling are not correlated with N₆₀ values.
- 3S = Same as 2S, but using a 3.0 inch O.D. split spoon sampler.
- TR = Top of rock
- W = Initial water level measured during drilling
- Water level measured at completion of drilling

Classification Test Data

Gradation (as defined on Description of Soil Terms):

GR	=	% Gravel
SA	=	% Sand
SI	=	% Silt
CL	=	% Clav

Atterberg Limits:

LL	=	Liquid limit
PL	=	Plastic limit
PI	=	Plasticity Index
WC	=	Water content (%)

PROJECT: <u>MAR-529-2.59 FDR</u> DRILLIN TYPE: <u>ROADWAY</u> SAMPLI	g firm / op Ng firm / l(DGGER: ODOT / CAREY	DRI HAI	ILL RIG MMER:	: <u>CM</u> CME	E 55 T AUTO	RUCK MATIO	к С	STA ALIG	TION GNME	/ OFF	SET:		SR 52	' 29	E	XPLOR B-001	ATION ID 1 -0-20
PID: 112818 SFN: DRILLIN	G METHOD:	2.25" HSA	CAI	LIBRAT	ION DATE:	4	4/15/2	0	ELE	VATIO	ON: _	975	.0 (MS	SL)	EOB:	7.5 ft.		PAGE
START: <u>3/9/21</u> END: <u>3/9/21</u> SAMPLI	NG METHOD):SPT	ENE	ERGY F	RATIO (%):		83.6		LAT	/LON	NG: _		40	.5494	72, -83	.078082		1 OF 1
MATERIAL DESCRIPTION	ELEV.	DEPTHS SPT/	N	REC	SAMPLE	ΗP	C	GRAD	ATIO	N (%))	ATT	ERBE	ERG		ODOT	SO4	BACK
AND NOTES	975.0	RQD	N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.2' - ASPHALT (14.0")	973.8																	
0.3' - AGGREGATE BASE (4.0")	973.5																	9 L
STIFF, GRAY TO BROWNISH GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	972.0	-2 -4 - 5	13	83	SS-1	2.00	5	4	10	46	35	40	25	15	24	A-6a (10)	100	
STIFF TO HARD, GRAY TO BROWNISH GRAY CLAY , SOME SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		- 3 - 2 - 2 - 3 - 4 - 3 - 4	10	100	SS-2	1.50	2	2	10	35	51	53	21	32	25	A-7-6 (19)	-	
		-5 -3 - 3 - 4	10	33	SS-3	3.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	
	967.5		13	100	SS-4	4.25	-	-	-	-	-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

D	PROJECT:	MAR-529-2.59 FDR	DRILLING	FIRM / OP	ERATOR:		AREY			:CM	E 55 T AUTO	RUCK	:	STA		/ OFF	SET:		SR 5	, 29	E	XPLOR B-002	ATION ID 2 -0-20
	PID: <u>112818</u> START: <u>3/9/2</u>	_ SFN:	DRILLING	METHOD: G METHOD	2.	25" HSA SPT			LIBRAT ERGY F	ION DATE:		4/15/2	0	ELE	VATIC	DN: _	975.	.0 (MS 40	SL)	EOB: 0083	7.5 ft.		PAGE 1 OF 1
	MATERIA	AL DESCRIPTION ND NOTES		ELEV. 975.0	DEPTH	IS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE	HP (tsf)	GR (GRAD CS	ATIO	N (%) SI) CL	ATT	ERBE	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.2' - ASI	PHALT (14.0") GREGATE (MACA	DAM) BASE (4.0")		973.8 973.5							, ,												
BROWN TO FINE	ISH GRAY SILT AI SAND, TRACE FII	ND CLAY, SOME CO NE GRAVEL, MOIST.	ARSE	972.0		- 2 - 4 - - 3 -	1 5 4	13	0	AS-1	-	3	8	13	39	37	33	18	15	20	A-6a (10)	220	
STIFF TO DARK BF SAND, L	O VERY STIFF, DA ROWN SILTY CLA ITTLE FINE GRAV	RK GRAY TO BROW Y, LITTLE COARSE EL, DAMP TO MOIST	/N AND TO FINE T.			6 - 4 -	3 7 5	17	44	SS-2	1.50	20	7	10	35	28	34	18	16	21	A-6b (8)	-	
						- 5 - 5	5 5 7	17	100	SS-3	3.50	-	-	-	-	-	-	-	-	14	A-6b (V)	-	
				967.5	EOB-	8 _ 7 _	³ 12 ₉	29	100	SS-4	2.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJECT:	MAR-529-2.59	9 FDR	DRILLING	FIRM / OPE		ODOT /	CAREY	DRI	ILL RIG	: <u>CM</u>	E 55 T	RUCK	(STA	TION	/ OFF	SET:			1		EXPLORA	TION ID
K11	TYPE:	ROADWAY		SAMPLING	FIRM / LO	GGER:		ICLEISH		MMER:		AUTO	MAII	2		INME	NI: _			SR 52	29	L		PAGE
	PID: <u>112818</u>	SFN:			METHOD:	2				-IBRA I	ION DATE:		4/15/2	0	ELE	VATIC)N: _	979.	0 (MS	iL)	FOR:	7.5 ft	-	1 05 1
	START:3/	<u>3/21</u> END:	3/3/21	SAMPLING	METHOD	:	SPT		ENE	ERGY	Ratio (%):		83.6		LAT	/LON	1G: _		40	.54947	71, -83	.072329		1011
	MATE	RIAL DESCRIPTI	ION		ELEV.		ЦС	SPT/	N	REC	SAMPLE	HP	0	GRAD	ATIO	N (%))	ATT	ERBE	RG		ODOT	SO4	BACK
		AND NOTES			979.0			RQD	1 1 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASI	PHALT (12.0")	E (6.0")			978.0		 - 1 -	-																
				-	977.5		⊢ r																-	9 - 004 838000
CLAY, SO TRACE F	OME TO "AND" INE GRAVEL, I	COARSE TO FIN DAMP TO MOIST	IE SAND,				- 2 -	3 3 5	11	78	SS-1	1.50	2	18	18	40	22	34	18	16	19	A-6b (8)	250	
							- 3 - - 4 -	2 3 6	13	89	SS-2	2.00	7	8	15	35	35	35	17	18	17	A-6b (10)	-	
							- 5 -	4 6 7	18	100	AS-3	2.50	-	-	-	-	-	-	-	-	14	A-6b (V)	-	X X X X X X X X X X X X X X X X X X X
					971.5	FOR	- 6 - - - 7 -	4 3 3	8	78	SS-4	1.50	-	-	-	-	-	-	-	-	15	A-6b (V)	-	
						LOD																		

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: <u>MAR-529-2.59 FDR</u> DRILI TYPE: <u>ROADWAY</u> SAMF	LING FIRM / OPE PLING FIRM / LO	RATOR: ODOT / CAREY GGER: ODOT / MCLEISH	DRILL HAMN	l Rig: Mer:	CM CME	E 55 T AUTO	RUCK MATIC	;	STA ALIG	TION GNME	/ OFF NT:	SET:		SR 52	' 29	E	XPLOR	ATION ID 5 -0-20
PID: <u>112818</u> SFN: DRILI START: 3/3/21 END: 3/3/21 SAMF	LING METHOD: PLING METHOD:	2.25" HSA SPT	CALIE	BRATIO RGY R/	ON DATE: ATIO (%):		4/15/20 83.6)	ELE' LAT	VATIO	DN: _ NG:	980.	.0 (MS 40	L) .5495	EOB: 00, -83	7.5 ft.		PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 980.0	DEPTHS SPT/ RQD	N ₆₀ R	REC 8 (%)	SAMPLE ID	HP (tsf)	GR	RAD/ cs	ATIO FS	N (%) SI) CL	ATT LL	ERBE PL	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.8' - ASPHALT (9.0") 0.7' - AGGREGATE (MACADAM) BASE (9.0")	979.2																	
VERY STIFF, BROWNISH GRAY SILTY CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	977.0	$\begin{array}{c} 2 \\ -2 \\ -3 \\ -3 \\ -3 \\ -3 \\ -3 \\ -3 \\ $	13	89	SS-1	3.00	8	4	9	26	53	34	17	17	18	A-6b (11)	<100	
VERY STIFF, BROWN AND GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, LITTLE FINE GRAVEL, DAMP TO MOIST.		$\begin{array}{c} -3 \\ -4 \\ -4 \end{array}$	22 1	100	SS-2	3.50	12	8	11	37	32	30	17	13	16	A-6a (8)	-	
		-5 -7 -6 -12	29 1	100	SS-3	4.00	-	-	-	-	-	-	-	-	15	A-6a (V)	-	
	972.5	EOB 8 - 7 - 8 12	28 1	100	SS-4	4.00	-	-	-	-	-	-	-	-	16	A-6a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJECT:	MAR-529-2	2.59 FDR							ILL RIG	: <u>CM</u>	IE 55 T		< ~	STA		/ OFF	SET:		SD 50	' 20	E	EXPLOR B-009	ATION ID 9-0-20
RI	PID:	2818 SFN:			METHOD:	JOGER	2.25" HSA			LIBRAT	ION DATE:		4/15/2	0	ELE		DN: _	977.	.0 (MS	SK 52	EOB:	7.5 ft		PAGE
	START:	<u>3/3/21</u> END:	3/3/21	SAMPLING	i METHOD):	SPT		ENE	ERGY F	Ratio (%):		83.6		LAT	/ LON	1G:		40	.54945	53, -83	.066560		
	MA	ATERIAL DESCRI	PTION		ELEV.	DEPT	HS	SPT/	Neo	REC	SAMPLE	HP	(GRAD		N (%))	ATT	ERBE	RG			SO4	BACK
		AND NOTES			977.0	<u> </u>		RQD	00	(%)	ID	(tst)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLA33 (GI)	ppin	XXXXXXX
1.2' - ASI	PHALT (14.0	")			975.8		- 1 -																	
0.3' - AG	GREGATE E	BASE (4.0")			975.5	-	+ ,	d																9 E all
VERY ST COARSE	TIFF, DARK E TO FINE S	GRAY SILTY CLA AND, TRACE FINE	Y, LITTLE E GRAVEL, DAN	ЛР.	974.0		- 2 -	.4 5 5	14	89	SS-1	2.50	2	3	10	51	34	40	21	19	18	A-6b (12)	<100	
STIFF, D TRACE (DAMP TO	OARK GRAY COARSE TO O MOIST.	TO GRAY CLAY , FINE SAND, TRA	"AND" SILT, .CE FINE GRAV	EL,			- 3 - - - 4 -	2 3 4	10	83	SS-2	1.50	3	1	7	43	46	48	20	28	23	A-7-6 (17) -	
							- 5 -	3 3 3	8	83	SS-3	1.50	-	-	-	-	-	-	-	-	25	A-7-6 (V)	-	
					969.5	EOB-	- 7 -	3 3 5	11	89	SS-4	1.50	-	-	-	-	-	-	-	-	25	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529-2.59 F ROADWAY	DR	DRILLING SAMPLING	FIRM / OPE FIRM / LC	ERATOR:	ODOT / ODOT / M	CAREY CLEISH	DRI HAN	ILL RIG MMER:	:CM	E 55 T AUTO	RUCK	()	STA ⁻ ALIG	TION NMEI	/ OFF NT:	SET:	;	SR 52	' 29	E)	(PLOR/ B-010	TION ID - 0-20
	PID: 112818	SFN:		DRILLING	METHOD:		2.25" HSA			IBRAT	ION DATE:		4/15/2	0	ELE	/ATIC	DN: _	977.	0 (MS	L)	EOB:	7.5 ft.		PAGE
	START: 3/3/	21 END:	3/3/21	SAMPLING	METHOD	:	SPT		ENE	ERGY F	ratio (%):		83.6		LAT	/ LON	IG:		40.	54948	31, -83	.065111		TOFT
	MATER	IAL DESCRIPTIO	N		ELEV.		гнs	SPT/	N	REC	SAMPLE	HP		GRAD	ATIO	<mark>√ (%)</mark>		ATT	ERBE	RG		ODOT	SO4	BACK
	A	AND NOTES			977.0	DLI	mo	RQD	[™] 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
0.7' - AS	PHALT (8.0")				976.3																			
0.8' - AG	GREGATE (MAC	ADAM) BASE (10	.0")		975.5		- 1																	
VERY S GRAVEL	TIFF, GRAY SILT ., TRACE COARS	Y CLAY, LITTLE F E TO FINE SAND,	INE , MOIST.		974.0		- 2 -	4 4 5	13	78	SS-1	3.00	13	3	7	38	39	37	19	18	24	A-6b (11)	<100	
STIFF, G FINE SA	GRAY CLAY , SOM ND, TRACE FINE	IE SILT, LITTLE C GRAVEL, MOIST	OARSE TO		972.5		- 4 -	3 4 5	13	100	SS-2	2.00	8	6	9	35	42	43	19	24	23	A-7-6 (14)	-	
VERY S COARSE	tiff, brown Sil E to fine sand,	T AND CLAY , LIT TRACE FINE GR	ITLE AVEL, DAM	IP.			- 5 -	4 6 8	20	89	SS-3	4.00	-	-	-	-	-	-	-	-	14	A-6a (V)	-	
					969.5	—ЕОВ—	- 7 -	6 8 10	25	100	SS-4	4.00	-	-	-	-	-	-	-	-	14	A-6a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

		MAR-529-2.59 FDR		FIRM / OF		ODOT /	CAREY	DR		:CM	E 55 T	RUCK		STA		/ OFF	SET:			,	E	EXPLOR B-01	ATION ID 3-0-20
K11)	1 YPE:				JGGER: _ (ICLEISH				AUTO		ز م			- INI:	079	0 / 14	SR 52	29 EOD:	L		PAGE
	PID. 112010				· <u> </u>	20 ПОА 901						4/15/2 83.6	0			JN	970.	0 (IVIC). 10	<u>5404</u>	EUD. 33 83	<u> </u>	·	1 OF 1
						JF I						03.0	חאםי		/ LOI	<u>vo.</u>	ATT		-0494	33, - 03	.000770		
	IVIAIER	NND NOTES		078 0	DEPTI	HS	ROD	N ₆₀	(%)		⊓P (tef)	GR			1 N (70)				wc	ODOT CLASS (GI)	SO4	BACK FILL
1.0' - ASF 0.5' - AG	PHALT (12.0") GREGATE BASE	(6.0")		977.0	_				(70)		(131)		00	10	01	UL	LL			wo			
STIFF, D. COARSE -WOOD	ARK GRAY SILT SAND, TRACE I FRAGMENTS IN	Y CLAY, SOME FINE TO FINE GRAVEL, DAMP. V SS-1		975.0	-	- 2 -	3 3 3	8	67	SS-1	1.50	7	18	10	33	32	40	21	19	17	A-6b (10)	150	
SOFT TC SILT, SO GRAVEL -TRACE) STIFF, DARK G ME COARSE TC , MOIST. E ORGANICS IN S	RAY TO GRAY CLAY , " FINE SAND, TRACE FI SS-2	AND" NE			- 3 - - 4 -	2 2 3	7	67	SS-2	1.50	8	8	17	37	30	43	20	23	30	A-7-6 (12)	-	
						- 5 -	1 2 1	4	100	SS-3	0.50	-	-	-	-	-	-	-	-	29	A-7-6 (V)	-	
				970.5	EOB-	- 7 -	1 2 3	7	100	SS-4	0.50	-	-	-	-	-	-	-	-	25	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: TYPE: PID:11	MAR-529- ROADW 12818 SFN:	2.59 FDR /AY	DRILLING SAMPLINC DRILLING	FIRM / OPE FIRM / LC METHOD:	 GGER:(;	ODOT / ODOT / M 2.25" HSA	CAREY	DR HAI CAI	ILL RIG MMER: LIBRAT	: <u>CM</u> CME	IE 55 T AUTO	RUCK MATIC 4/15/2	2 0	STAT ALIG ELEV	ION / C IMENT ATION	FFSET	9.0 (MS	SR 52 SL)	' 29 EOB:	E	XPLOR B-014	ATION ID I-0-20 PAGE
START:	START: <u>3/3/21</u> END: <u>3/3/21</u> SAMPLIN			AMPLING METHOD:		SPT		ENERGY RATIO (%):		83.6			LAT /	LONG	40.5494			56, -83.059334			1 OF 1	
M	ATERIAL DESCRI AND NOTES	PTION ;		ELEV. 979.0	DEPT	ΉS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	CS CS	ATION FS	(%) si c	ATI	PL	ERG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASPHALT (12. 0.5' - AGGREGATE	0") (MACADAM) BAS'	E (6.0")		978.0			-															
MEDIUM STIFF, BR COARSE SAND, SO	OWN SILTY CLAY ME FINE GRAVEL	I, SOME FINE TO , MOIST.	2 C	976.0		- 2 -	14 6 5	15	100	SS-1	0.75	35	15	7	22 2	36	19	17	21	A-6b (4)	120	
MEDIUM DENSE, BI SAND, SILT, AND C	ROWN STONE FR I LAY , MOIST.	AGMENTS WITH		974.5		- 3 - - 4 -	3 6 14	28	11	SS-2	-	-	-	-		-	-	-	13	A-2-4 (V)	-	
SOFT, BROWN SIL SAND, SOME FINE -LIMESTONE FRAG	FY CLAY , SOME F GRAVEL, DAMP. GMENTS IN SS-3	INE TO COARS				- 5 -	8 8 7	21	0	SS-3	-	22	14	10	31 2	3 34	17	17	14	A-6b (7)	-	
				971.5	FOR	- 7 -	6 6 9	21	44	SS-4	0.50	-	-	-		-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DR TYPE: ROADWAY SAU PID: 112818 SFN: DI	RILLING MPLING RILLING	FIRM / OPE 6 FIRM / LOO METHOD:	RATOR: ODOT GGER: ODOT / I 2.25" HS	/ CAREY MCLEISH A	DR HA CA	ILL RIG MMER: LIBRAT	:CM CME ION DATE:	E 55 T AUTO	RUCK MATIO 4/15/2	() 0	STA ALIC ELE	TION GNME VATIO	:/ OFI :NT: _ :NN: _	SET: 982	.0 (MS	SR 52	' 29 EOB:	E	XPLOR B-017	ATION ID 7-0-20 PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	IVIPLING	ELEV. 982.0	DEPTHS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	83.6 GR	GRAD CS		7 LOI N (% SI	NG:	ATT LL	ERBE PL	ERG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.6' - ASPHALT (8.0") 0.9' - AGGREGATE (MACADAM) BASE (10.5")		981.4 980.5		-																
MEDIUM STIFF, GRAY SANDY SILT , SOME CLAY, TRACE FINE GRAVEL, MOIST.		979.0	- 2 -	4 4	11	100	SS-1	1.00	5	9	14	43	29	29	21	8	23	A-4a (7)	130	
STIFF, GRAY TO BROWN CLAY , SOME SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.		977.5	3 - 4 -	- ³ 4 6	14	100	SS-2	2.00	8	7	12	35	38	41	18	23	21	A-7-6 (13)	-	
VERY STIFF, BROWN TO BROWNISH GRAY SANDY SILT , LITTLE CLAY, TRACE FINE GRAVEL, DAMP.			- 5 -	-6 4 7	15	100	SS-3	2.50	-	-	-	-	-	-	-	-	14	A-4a (V)	-	
		974.5	— ЕОВ — С - С - С - С - С - С - С - С - С - С	-6 10 12	31	100	SS-4	3.00	-	-	-	-	-	-	-	-	14	A-4a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRILLIN TYPE: ROADWAY SAMPLIN PID: 112818 SFN: DRILLIN	G FIRM / OPEI IG FIRM / LOG G METHOD: _	RATOR: ODOT / CAR GGER: ODOT / MCLEI 2.25" HSA	REY C EISH H	ORILL RIG IAMMER: CALIBRAT	::CM CME ION DATE:	E 55 T AUTO	RUCK MATIC 4/15/20	;	STATIC ALIGNM ELEVA1	N / OF IENT: ION:	FSET:	S .0 (MSL	ir 529) E	, 9 EOB: _	E	XPLOR/ B-018	TION ID -0-20 PAGE 1 OF 1
START: <u>3/3/21</u> END: <u>3/3/21</u> SAMPLIN MATERIAL DESCRIPTION AND NOTES	G METHOD: ELEV. 981.0	DEPTHS SPT/ RQD		NERGY I	SAMPLE	HP (tsf)	83.6 GR	RADA CS F	LAT / LO TION (' s s si	DNG: _ %) CL	ATT	40.5 ERBEF	49426 RG PI	<u>6, -83.0</u> wc	ODOT CLASS (GI)	SO4 ppm	BACK
0.7' - ASPHALT (8.0") 0.8' - AGGREGATE (MACADAM) BASE (10.0")	980.3																
STIFF, GRAY TO BROWN SILT , SOME CLAY, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	978.0	- 2 - 4	4 11	89	SS-1	1.50	6	6 1	2 50	26	29	20	9	19	A-4b (8)	<100	
STIFF TO VERY STIFF, BROWNISH GRAY SILTY CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		-3 -2 -4 -3	3 10) 78	SS-2	2.00	3	4 1	1 41	41	40	18	22	22	A-6b (13)	-	
-LIMESTONE FRAGMENTS IN SS-3		_ 5 _ 3 _ 6	4 10 3	56	SS-3	1.50	-	-		-	-	-	-	20	A-6b (V)	-	
	973.5	- 0 - 2 - 7 - 5	5 18 8	3 100	SS-4	3.00	-	-		-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING
	PROJECT:	MAR-52	9-2.59 FDR		FIRM / OP	ERATOR:	ODOT	CAREY	DR	ILL RIG	: <u>CM</u>	IE 55 T	RUCK		STA	TION	/ OFF	SET:			'	E	XPLOR	ATION ID -0-20
K11	I YPE: PID: 1128	ROAD	WAY			GGER:	25" HSA	ACLEISH		MMER: I IBRAT		AUTO	MATIC 4/15/2	<u>;</u> ח		INMEI	NI:	979 (3R 52	9 FOB'	L		PAGE
	START:	3/4/21 ENI	D: <u>3/4/21</u>	SAMPLING	6 METHOD):	SPT		EN	ERGY F	RATIO (%):		83.6	•	LAT	/ LON	IG:	0.0.0	40.5	- <u>/</u> 54938	1, -83.	.049251		1 OF 1
	MAT	TERIAL DESC	RIPTION		ELEV.	DEPT	нs	SPT/	N	REC	SAMPLE	HP	0	RAD	ATIO	N (%)		ATTE	RBE	RG		ODOT	SO4	BACK
		AND NOTE	S		979.0			RQD	™ 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.2' - ASI	PHALT (14.0")				977.8		- 1 -																	
0.3' - AG	GREGATE BA	ASE (4.0")			977.5		-																	A Fall
STIFF, D TO FINE	ARK GRAY S SAND, TRAC	ILT AND CLA E FINE GRAV	Y , LITTLE COARS EL, DAMP.	E	976.0		- 2 -	-3 4 4	11	100	SS-1	1.50	10	6	17	43	24	32	20	12	18	A-6a (7)	230	
SOFT TO SOME C MOIST.	O STIFF, BRO OARSE TO FI	WNISH GRAY INE SAND, TR	CLAY , "AND" SIL ACE FINE GRAVE	T, EL,			- 3 - - - 4 -	2 3 4	10	100	SS-2	0.50	1	2	20	41	36	48	15	33	21	A-7-6 (18)	-	
							- 5 -	2 2 2	6	50	SS-3	0.50	-	-	-	-	-	-	-	-	22	A-7-6 (V)	-	
					971.5	EOB-	- 6 - - - 7 -	2 3 6	13	89	SS-4	1.50	-	-	-	-	-	-	-	-	25	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJECT:		MAR-529-2.	.59 FDR	DRILLING	FIRM / OP	ERATOR:	ODOT /	CAREY	DR	ILL RIG	: CN	E 55 T	RUCK		STA		/ OFF	SET:			1	E		ATION ID
Rfil	TYPE:		ROADWA	Y	SAMPLING	FIRM / LC	DGGER:	ODOT / N	ICLEISH	HAI	MMER:	CME	AUTO	MATIO	2	ALIC	SNME	NT: _			SR 52	29	L	D-024	2-0-20
	PID: 112	818	SFN:		DRILLING	METHOD:		2.25" HSA	۱	CAI	IBRAT	ION DATE:		4/15/2	0	ELE	VATIO	DN:	979.	.0 (MS	SL)	EOB:	7.5 ft		PAGE
	START:	3/3/21	END:	3/3/21	SAMPLING	METHOD):	SPT		EN	ERGY F	RATIO (%):		83.6		LAT	/LON	IG:		40	.5494	19, -83.	.047807		1 OF 1
	MA	TERIAL	DESCRIP	TION		ELEV.		ue	SPT/	N	REC	SAMPLE	ΗP	0	GRAD	ATIO	N (%)	ATT	ERBE	RG		ODOT	SO4	BACK
		AND	NOTES			979.0	DEFI	по	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
0.8' - ASI	PHALT (10.0"	')				978.2			-																
0.7' - AG	GREGATE (N	ACADA	M) BASE	(8.0")	\otimes	977.5		- 1 -																	
VERY ST FINE TO	TIFF, DARK O COARSE SA	GRAY S I ND, LIT	LT AND C TLE FINE	LAY, SOME GRAVEL, MOI	ST.	976.0		- 2 -	944	11	83	SS-1	2.75	13	11	10	40	26	32	20	12	24	A-6a (7)	240	
VERY ST COARSE	TIFF, BROWN E TO FINE SA	NISH GF ND, LIT	RAY SILTY TLE FINE	GRAVEL, MOI	ST.	010.0		- 3 - - - 4 -	- ³ 4 5	13	0	AS-2	-	12	7	9	38	34	40	16	24	28	A-6b (13)	-	
								- 5 -	3 5 6	15	100	SS-3	3.00	-	-	-	-	-	-	-	-	17	A-6b (V)	-	
						971.5	EOB-	- 7 -	4 4 4	11	0	AS-4	-	-	-	-	-	-	-	-	-	22	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJEC	T:	MAR-529-2.	59 FDR	DRILLING	FIRM / OP	ERATOR:	ODOT /	CAREY	DRI	LL RIG	CM	E 55 T	RUCK	(STA	TION	/ OFF	SET:						ATION ID
Rii)	TYPE:		ROADWA	Y	SAMPLING	FIRM / LC	GGER:	ODOT / M	CLEISH	_ HAN	MMER:	CME	AUTO	MATIO	2	ALIC	SNME	NT: _			SR 52	29		D-02;	0-0-20
	PID:	112818	SFN:		DRILLING	METHOD:		2.25" HSA		CAL	IBRAT	ION DATE:	4	4/15/2	0	ELE	VATIO	DN:	975	.0 (MS	SL)	EOB:	7.5 f	t.	PAGE
	START:	3/4/2	END:	3/4/21	SAMPLING	METHOD	:	SPT			ERGY F	RATIO (%):		83.6		LAT	/LON	_ IG: _		40	.54942	25, -83	.043482		1 OF 1
		MATERIA	L DESCRIP	TION		ELEV.		110	SPT/	N	REC	SAMPLE	HP	(GRAD	ATIO	N (%)	ATT	ERBE	RG		ODOT	SO4	BACK
		Al	ID NOTES			975.0	DEPT	пэ	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)) ppm	FILL
1.2' - ASI	PHALT (1	4.0")				973.8																			
0.3' - AG	GREGAT	EBASE (4.0")			_ 973.5		-																	9 E all
STIFF TO COARSE TO MOIS	D VERY S TO FINE T.	STIFF, BR SAND, T	OWN SILTY RACE FINE	CLAY , LITTLE GRAVEL, DAM				- 2 -	8 5 5	14	56	SS-1	2.00	8	10	8	38	36	38	20	18	10	A-6b (11)	150	
								- 4 -	4 5 8	18	100	SS-2	2.00	5	7	11	33	44	38	19	19	18	A-6b (12)) -	
								- 5 -	8 8 10	25	100	SS-3	3.00	-	-	-	-	-	-	-	-	15	A-6b (V)	-	
						967.5	—EOB—	- 7 -	6 8 11	26	100	SS-4	4.00	-	-	-	-	-	-	-	-	15	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRIL TYPE: ROADWAY SAM PID: 112818 SFN: DRIL	LING FIRM PLING FIR LING MET	M / OPERATOR: RM / LOGGER: THOD:	ODOT / 0 ODOT / M0 2.25" HSA	CAREY	DRI HAM CAL	ILL RIG MMER: _IBRAT	:CM CME ION DATE:	E 55 T AUTO	RUCK MATIC 4/15/2	2 0	STA ALIC ELE	TION GNMEI VATIC	/ OFF NT:	SET: 972.	0 (MS	SR 52 iL)	' 29 EOB:	E	KPLOR B-026	ATION ID 5-0-20 PAGE
START: <u>3/3/21</u> END: <u>3/3/21</u> SAM MATERIAL DESCRIPTION AND NOTES	PLING ME	LEV. DEP	SPT THS	SPT/	N ₆₀	ERGY F	SAMPLE	HP (tsf)	83.6 GR	GRAD		/ LON N (%)	IG:	ATT	40 ERBE PI	.5494 RG PI	57, -83 wc	.042042 ODOT CLASS (GI)	SO4 ppm	BACK
0.8' - ASPHALT (10.0") 0.7' - AGGREGATE (MACADAM) BASE (8.0")	97 97 97	7 <u>1.2</u> 70.5				(70)		(101)				0.	01							
STIFF TO VERY STIFF, BROWNISH GRAY TO GRAY SILTY CLAY , LITTLE COARSE TO FINE SAND, TRACE TO LITTLE FINE GRAVEL, DAMP TO MOIST.			- 2 -	11 8 4	17	67	SS-1	2.50	11	11	9	37	32	38	20	18	17	A-6b (10)	210	
	96	67.5	- 4 -	2 3 5	11	83	SS-2	2.00	4	5	10	37	44	39	18	21	23	A-6b (12)	-	
DENSE TO VERY DENSE, GRAY GRAVEL WITH SAND AND SILT , TRACE CLAY, MOIST TO WET. -ASPHALT FRAGMENTS IN SS-3			- 5 -	26 24 26	70	89	SS-3	-	52	15	7	- 26	6 -	-	-	-	14	A-2-4 (V)	-	
		64.5 EOB	- 7 -	7 10 12	31	22	SS-4	-	-	-	-	-	-	-	-	-	16	A-2-4 (V)	-	

NOTES: GROUNDWATER ENCOUNTERED INITIALLY @ 6.0'

PROJECT: <u>MAR-529-2.59 FDR</u> DRILL TYPE: <u>ROADWAY</u> SAMPI PID: 112818 SEN: DRILL	NG FIRM / OF .ING FIRM / LO NG METHOD:	ERATOR: ODOT / CAREY OGGER: ODOT / MCLEISH 2.25" HSA		RILL RIG MMER: LIBRAT	:CM CME ION DATE:	<u>E 55 T</u> AUTO	RUCK MATIO 4/15/2	: C 0	STA ALIC ELE		/ OFf NT: _ ON:	FSET:	.0 (MS	SR 53	' 29 EOB:	E	XPLOR B-029	ATION ID 9-0-20 PAGE
START: <u>3/4/21</u> END: <u>3/4/21</u> SAMP	ING METHOD):	EN	IERGY I	RATIO (%):		83.6	•	LAT	/LOI	NG: _	0.0	40).5488	28, -83	.037810		1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 973.0	DEPTHS SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	GRAD	ATIO FS	N (% si) CL	ATT LL	ERB	ERG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASPHALT (12.0") 0.5' - AGGREGATE BASE (6.0")	972.0 971.5																	
VERY STIFF, BROWN SILTY CLAY , SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, DAMP.	970.0	-2 - 3 - 3 4 5	13	67	SS-1	3.50	5	12	9	43	31	38	21	17	19	A-6b (11)	350	
HARD, BROWN TO BROWNISH GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		-4 - 69	21	100	SS-2	4.50	7	8	11	38	36	32	18	14	16	A-6a (9)	-	
		- 5 - 6 - 8 9	24	100	SS-3	4.50	-	-	-	-	-	-	-	-	17	A-6a (V)	-	
	965.5	- 6 - 7 - 8 12	28	100	SS-4	4.00	-	-	-	-	-	-	-	-	18	A-6a (V)	-	

NOTES: GROUNDWATER ENCOUNTERED INITIALLY @ 4.0'

	PROJEC	T:	MAR-529-2	.59 FDR	DRILLING	FIRM / OPE	RATOR:	ODOT /	CAREY		LL RIG	:CM	E 55 T	RUCK		STA	TION	/ OFF	SET:				E		ATION ID
Rii)	TYPE:		ROADWA	Y	SAMPLING	FIRM / LO	GGER:	ODOT / M	CLEISH	HAI	MMER:	CME	AUTO	MATIO)	ALIC	SNME	NT: _			SR 52	29	L	D-030	-0-20
	PID:	112818	SFN:		DRILLING	METHOD:	2	2.25" HSA		CAL	IBRAT	ION DATE:		4/15/2	0	ELE	VATIO	ON:	973.	.0 (MS	SL)	EOB:	7.5 ft	.	PAGE
	START:	3/2/21	END:	3/2/21	SAMPLING	METHOD:		SPT		EN	ERGY F	RATIO (%):		83.6		LAT	/LON	NG:		40	.54852	22, -83	.036428		1 OF 1
		MATERIAL	DESCRIP	TION	-1	ELEV.		110	SPT/		REC	SAMPLE	HP	(RAD	ATIO	N (%)	ATT	ERBE	RG		ODOT	S04	BACK
		AN	D NOTES			973.0	DEPT	HS	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
1.0' - ASF	PHALT (12	2.0")		(0.011)		972.0					, ,														
0.5' - AG	GREGATE	= (MACAD	AM) BASE	(6.0")	\longrightarrow	971.5																			Pg Land
VERY ST COARSE GRAVEL	IFF, BRO TO FINE , MOIST.	own Silt ' Sand, Tf	(CLAY , LIT RACE TO LI	ITLE TO SOME				- 2 -	4 4 6	14	0	AS-1	-	12	18	8	34	28	37	21	16	23	A-6b (8)	110	
						968.5		- 4 -	3 5 7	17	100	SS-2	3.00	7	7	11	36	39	39	19	20	19	A-6b (12)	-	
VERY ST TRACE F	TFF, BRO TINE GRA	WN SANE VEL, DAM	P. SILT, LIT P.	TTLE CLAY,				- 5 -	7 9 10	26	100	SS-3	4.00	-	-	-	-	-	-	-	-	14	A-4a (V)	-	
						965.5	—ЕОВ—	- 7 -	11 10 14	33	100	SS-4	4.00	-	-	-	-	-	-	-	-	15	A-4a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRILL TYPE: ROADWAY SAMPI	.ING FIRM / OPP PLING FIRM / LC	BRATOR: ODOT / CAREY	DRILL HAMM	RIG: 1ER:	CME 55 T	RUCK	ST/	ATION GNME	/ OFF NT: _	SET:	SI	' R 529	E	EXPLOR B-03	ATION ID 3-0-20
PID: <u>112818</u> SFN: DRILL START: <u>3/4/21</u> END: <u>3/4/21</u> SAMP	ING METHOD: PLING METHOD	2.25" HSA :SPT	_ CALIB _ ENER	RATION D. GY RATIO	ATE: (%):	4/15/20 83.6			DN: _ \G: _	973.0	0 (MSL) 40.54	EO 17474, -	3: <u>7.5 ft</u> 33.032303		1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 973.0	DEPTHS SPT/	N ₆₀ R	EC SAM	LE HP			DN (%)) Cl			G N WO	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASPHALT (12.0")	972.0							0.	01						
VERY STIFF, GRAY SANDY SILT , SOME CLAY, TRACE FINE GRAVEL, DAMP.	970.0	-2 - 8 - 8 - 8 - 8 - 8 - 8 - 8 - 8 - 8 -	13	78 SS-	1 2.50	3 6	12	43	36	32	22 1	0 17	A-4a (8)	360	
VERY STIFF TO HARD, BROWN CLAY , SOME SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		-3 -3 -3 -4 -3 -4 -7	15 8	39 SS-	2 2.50	2 5	10	35	48	46	21 2	25 24	A-7-6 (15) -	
		-5-6 - 5 - 6	15 1	00 SS-	3 2.50		-	-	-	-	-	- 15	A-7-6 (V)	-	
	965.5	EOB	22 1	00 SS-	4 4.50		-	-	-	-	-	- 15	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJEC	СТ:	CAREY	DRI	ILL RIG	:CM	E 55 T	RUCK	<u> </u>	STAT	FION .	/ OFFS	ET:				E	XPLOR	ATION ID						
Rii) TYPE: ROADWAY SAMPLING FIRM / LOGGER: OD										HAN	MMER:	CME	AUTO	MATIC	2	ALIG	NME	NT:		S	R 52	9		B-034	-0-20
	PID:	112818	SFN:		DRILLING	METHOD:		2.25" HSA		CAL	IBRAT	ION DATE:	4	4/15/2	0	ELE\	ATIC	N:	972.0	(MSL))	EOB:	7.5 ft.		PAGE
	START:	3/2/2	1 END:	3/2/21	SAMPLING		:	SPT		ENE	ERGY F	RATIO (%):		83.6		LAT	LON	G:		40.5	4716	3, -83.	.030917		1 OF 1
		MATERIA	L DESCRIP	PTION		ELEV.		FLIO	SPT/		REC	SAMPLE	HP	Ģ	GRAD	ATION	۱ (%)		ATTE	RBEF	RG		ODOT	SO4	BACK
		Al	ND NOTES			972.0	DEP	IHS	RQD	N ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	wc	CLASS (GI)	ppm	FILL
1.2' - ASF	PHALT (1	4.0")				970.8																			
).3' - AG	GREGAT	E (MACA	DAM) BASE	(4.0")	XXX	_ 970.5		- I																	9 - ant
SOFT TC BROWNI TO FINE) MEDIUI SH GRA SAND, L	M STIFF, I Y CLAY , " ITTLE FIN	DARK GRAY AND" SILT, IE GRAVEL,	(TO LITTLE COARS DAMP TO	E			- 2 -	4 4 4	11	0	AS-1	-	14	6	9	42	29	41	26	15	16	A-7-6 (9)	370	
VIOIST.								- 4 -	2 3 3	8	56	SS-2	0.50	17	4	7	36	36	51	19 3	32	22	A-7-6 (17)	-	
								- 5 -	2 3 3	8	100	SS-3	1.00	-	-	-	-	-	-	-	-	23	A-7-6 (V)	-	X V V V
						964.5	FOP	- 7 -	3 3 3	8	28	SS-4	0.50	-	-	-	-	-	-	-	-	25	A-7-6 (V)	-	
							LOB-																		

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: <u>MAR-529-2.59 FDR</u> DRILLIN TYPE: <u>ROADWAY</u> SAMPLI	g firm / Op Ng firm / L(ERATOR: <u>ODOT / CAREY</u> DGGER: <u>ODOT / MCLEISH</u>	DF HA	RILL RIG AMMER:	: CM	E 55 T AUTO	RUCK))	STA ALIC	TION	/ OFF	SET:		SR 52	' 29	E	KPLOR/ B-037	ATION ID 7-0-20
PID: <u>112818</u> SFN: DRILLIN START: <u>3/8/21</u> END: <u>3/8/21</u> SAMPLI	G METHOD:	2.25" HSA ::SPT		ALIBRAT IERGY F	ION DATE: RATIO (%):		4/15/2 83.6	0	ELE LAT	VATIO 101 /)N: _ NG: _	976.	0 (MS. 40	L) .5461:	EOB: <u>31, -83</u>	7.5 ft. .026817		1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 976.0	DEPTHS SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	CS	ATIO FS	N (% si) CL	ATT	ERBE PL	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.8' - ASPHALT (10.0") 0.7' - AGGREGATE BASE (8.0")	975.2 974.5					, ,												
STIFF TO VERY STIFF, DARK GRAY TO BROWNISH GRAY CLAY , SOME TO "AND" SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.		-2 - 3 - 3 - 3 - 3 - 3 - 4	10	83	SS-1	2.00	3	5	10	52	30	41	26	15	26	A-7-6 (10)	390	
	971.5	- $ -$	11	100	SS-2	2.50	4	3	8	35	50	51	21	30	28	A-7-6 (18)	-	
SOFT, DARK GRAY TO BROWNISH GRAY CLAY , SOME TO "AND" SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.		-5 -2 - 3 - 3	8	100	SS-3	0.50	-	-	-	-	-	-	-	-	29	A-7-6 (V)	-	
	968.5		7	44	SS-4	0.50	-	-	-	-	-	-	-	-	21	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	-2.59 FDR /AY	DRILLING	FIRM / OPE G FIRM / LO	RATOR: GGER:	ODOT / ODOT / M	CAREY CLEISH	DRI HAN	LL RIG MMER:	:CM	E 55 T AUTO	RUCK MATIO	()	STA ⁻ ALIG	TION /	OFFSI T:	ET:	SR 5	' 29	[XPLOR B-03	ATION ID 3 -0-20	
	PID: 112818	SFN:		DRILLING	METHOD:		2.25" HSA		CAL	IBRAT	ION DATE:		4/15/2	0	ELE	/ATIO	N:9	76.0 (I	MSL)	EOB:	7.5 ft.		PAGE
	START:3/2	/21 END:	3/2/21	SAMPLIN	G METHOD:		SPT		ENE	ERGY F	RATIO (%):		83.6		LAT	/LON	G:		40.5458	20, -83	.025424		TOFT
	MATER	NAL DESCR	IPTION		ELEV.	DEP	THS	SPT/	N	REC	SAMPLE	ΗP	0	GRAD	ATIO	۷ (%)	A	TTER	BERG		ODOT	SO4	BACK
		AND NOTES	;		976.0	DLI		RQD	• 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL L	L PI	- PI	WC	CLASS (GI)	ppm	FILL
1.0' - AS	SPHALT (12.0")				975.0																		
0.5' - AG	GREGATE BASE	(6.0")			974.5																		
MEDIUN SILT, WI	/I DENSE, GRAY ET.	GRAVEL WI	FH SAND , TRAC	E A A A A A A A A A A A A A A A A A A A	973.0		- 2 -	7 4 4	11	17	SS-1	-	-	-	-	-		-	-	13	A-1-b (V)	310	
SOFT TO AND CLA FINE GR	o medium stiff Ay , little coa Ravel, damp.	, dark gra RSE to fine	AY TO GRAY SIL ∃ SAND, LITTLE	Т			- 4 -	4 3 4	10	44	SS-2	0.50	-	-	-	-		-	-	12	A-6a (V)	-	
					970.0		- 5 -	4 4 5	13	67	SS-3	1.00	13	6	6	36	39 3	7 22	2 15	15	A-6a (10)	-	
STIFF, E COARSE	BROWNISH GRA E TO FINE SAND	Y CLAY , SOM , TRACE FIN	VE SILT, LITTLE	ST.	968.5	—FOB—	- 7 -	3 3 4	10	100	SS-4	1.50	5	3	8	33	51 5	1 22	2 29	27	A-7-6 (18)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRIL TYPE: ROADWAY SAM	ILLING FIRM / OPE MPLING FIRM / LO	RATOR: <u>ODOT / CAREY</u> GGER: ODOT / MCLEISH	DRILL RIG HAMMER:	: <u>CM</u> CME	E 55 TF AUTON		STATION / (FFSET:	SR 5	29	EXPL	DRATION ID 041-0-20
PID: 112818 SFN: DRIL START: 3/4/21 END: 3/4/21 SAM		2.25" HSA		ION DATE:	4	/15/20		974.0	0 (MSL) 40 5447	EOB:	7.5 ft.	PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 974.0	DEPTHS SPT/ RQD	N ₆₀ REC	SAMPLE	HP (tsf)	GRAD	ATION (%)		ERBERG	WC CLA	DOT S SS (GI) p	O4 BACK pm FILL
1.0' - ASPHALT (12.0") 0.5' - AGGREGATE BASE (6.0")	973.0 972.5											S C A
STIFF, GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.	971.0	$\begin{array}{c} -2 \\ -3 \\ -2 \\ -5 \\ -5 \\ -4 \end{array}$	13 89	SS-1	2.00	5 7	9 38 4	1 33	20 13	18 A-6	ia (9) 1	50 50 50 50 50 50 50 50
STIFF TO VERY STIFF, GRAY TO BROWNISH GRAY CLAY, SOME SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.	969.5	$\begin{array}{c} 3 \\ - \\ - \\ 4 \\ - \\ 8 \end{array}$	21 100	SS-2	4.00	3 7	11 34 4	5 43	20 23	22 A-7-	6 (14)	
VERY STIFF TO HARD, BROWN SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.		-5 -7 10 12	31 100	SS-3	4.00			-		15 A-6	a (V)	
	966.5	EOB	35 100	SS-4	4.50			-		13 A-6	a (V)	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DF TYPE: ROADWAY SA PID: 112818 SFN: DF	RILLING FIRM / OF AMPLING FIRM / L RILLING METHOD	PERATOR: ODOT / CAREY OGGER: ODOT / MCLEISH : 2.25" HSA	DRI HAM CAL	ill Rig Mmer: Librat	:CM CME ION DATE:	E 55 T AUTO	RUCK MATIC 4/15/2))	STA ALIG ELE	TION SNME VATIO	/ OFF NT: _ DN: _	-SET: 974	0 (MS	SR 52 iL)	' 29 EOB:	E	XPLOR B-042	ATION ID -0-20 PAGE
START: <u>3/2/21</u> END: <u>3/2/21</u> SA MATERIAL DESCRIPTION AND NOTES	AMPLING METHO	DEPTHS SPT/	ENE N ₆₀	ERGY F	SAMPLE	HP (tef)	83.6 GR	GRAD		/ LON N (%	NG:	ATT	40. ERBE	.5444: RG	33, -83 wc	.019935 ODOT CLASS (GI)	SO4 ppm	BACK
1.0' - ASPHALT (12.0") 0.5' - AGGREGATE (MACADAM) BASE (6.0")	973.0 972.5	1 -				(101)					02							
VERY STIFF, GRAY SILTY CLAY , SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, DAMP.	971.0	-2 - 5 5 6	15	67	SS-1	2.50	5	15	10	30	40	34	18	16	17	A-6b (9)	180	
VERY STIFF, GRAY TO BROWNISH GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		-4 -4 -4 -4 -4 -4 -4 -4	14	83	SS-2	3.00	15	9	11	34	31	32	17	15	17	A-6a (8)	-	
		-5 -4 - 6 - 6	17	100	SS-3	3.00	-	-	-	-	-	-	-	-	15	A-6a (V)	-	
	966.5		18	100	SS-4	2.50	-	-	-	-	-	-	-	-	16	A-6a (V)	-	

02019 RII STAND ODOT LOG SULF (8.5 X 11) - OH DOT.GDT - 4/9/21 14:19 - U:\GI8\PROJECTS\2020\W-20-161.GPJ

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529-2.5 ROADWAY	9 FDR	DRILLING SAMPLING	FIRM / OP 6 FIRM / LC	ERATOR: _)GGER: _ (/ ODOT DOT / M	CAREY CLEISH	DRI HAN	LL RIG MMER:	: <u>CN</u> CME	IE 55 T AUTO	RUCK MATIO	: C	STA ALIO	TION	/ OFF NT:	SET:		SR 52	' 29	E	XPLOR B-045	ATION ID 5 -0-20
	PID: 112818	SFN:		DRILLING	METHOD:	2	2.25" HSA		CAL	IBRAT	ION DATE:		4/15/2	0	ELE	VATIO	DN: _	976.	.0 (MS	SL)	EOB:	7.5 ft.		PAGE
	START: 3/8/21	END:	3/8/21	SAMPLING	METHOD):	SPT		ENE	ERGY F	ratio (%):		83.6		LAT	/LON	IG:		40	.5431	70, -83	.015916		TOFT
	MATERIA	L DESCRIPT	ION		ELEV.	DEPT	нs	SPT/	N	REC	SAMPLE	HP		GRAD	ATIO	N (%)	ATT	ERBE	RG		ODOT	SO4	BACK
	AN	D NOTES			976.0		110	RQD	1 1 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASPI	HALT (12.0")	6 O")			975.0																			
	NEGATE DAGE (-	974.5		+ -																	9 - 204 8380001
SILT AND SAND, TR	CLAY, LITTLE TO ACE FINE GRAVE	SOME COA L, DAMP TO	RSE TO FINE MOIST.				- 2 -	3 3 4	10	11	SS-1	-	-	-	-	-	-	-	-	-	16	A-6a (V)	390	
							- 4 -	3 5 6	15	100	SS-2	2.00	4	13	10	37	36	33	20	13	11	A-6a (9)	-	
							- 5 -	4 6 8	20	100	SS-3	2.00	7	7	11	35	40	31	18	13	25	A-6a (9)	-	
					968.5	EOB	- 7 -	6 7 12	26	100	SS-4	3.00	-	-	-	-	-	-	-	-	17	A-6a (V)	-	

02019 RII STAND ODOT LOG SULF (8.5 X 11) - OH DOT.GDT - 4/9/21 14:19 - U:\GI8\PROJECTS\2020\W-20-161.GPJ

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Dii	PROJEC TYPE:	T:	MAR-529-2. ROADWA	59 FDR Y		FIRM / OP G FIRM / LO	ERATOR:		CAREY	DR HAI	ILL RIG MMER:	:CM	E 55 T AUTO	RUCK MATIO		STA ALIO		/ OFF NT:	SET:		SR 52	' 29		EXPLOR	ATION ID 5 -0-20
	PID:	112818	SFN:	0/0/04		METHOD:		2.25" HSA		CAI	LIBRAT	ION DATE:		4/15/2)	ELE	VATIO	DN: _	979.	0 (MS	L)	EOB:	7.5 ft		PAGE 1 OF 1
	START:	3/2/21		3/2/21	SAMPLING):	SPI		EN				83.6					A T T	40.	54278	86, -83.	.014645		
		IVIA I ERIAL ANI	DESCRIP	TION		979.0	DEP	THS	RQD	N ₆₀	(%)	ID	HP (tsf)	GR	CS	FS	SI	CL			PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASF	PHALT (12	2.0") = (MACAD		(6.0")		978.0	-	1 -	-																
VERY ST COARSE	IFF, BRO	<u>= (MACAD</u> WN SILT SAND, TF	AM) BASE AND CLAY RACE FINE	, SOME GRAVEL, MOI	ST.	977.5		- 2 -	944	11	78	SS-1	2.50	2	8	13	38	39	34	19	15	22	A-6a (10)	410	
VERY ST SILTY CL TO FINE	TIFF TO H _AY , SOM SAND, D	ard, Brc 1e fine gi Amp.	WN TO BR RAVEL, LIT	OWNISH GRA TLE COARSE	Y			_ 4 -	2 4 5	13	100	SS-2	3.00	23	7	10	29	31	37	18	19	15	A-6b (9)	-	
								- 5 -	4 5 5	14	100	SS-3	3.00	-	-	-	-	-	-	-	-	15	A-6b (V)	-	
						971.5	EOB-	- - 7 -	3 7 8	21	100	SS-4	4.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

	PROJECT:	MAR-529-2.59 FDR		FIRM / OP	ERATOR: ODOT	/ CAREY	DR	ILL RIG	:CM	E 55 T	RUCK	(STA		/ OFF	SET:		00.50	'	E	XPLOR B-049	ATION ID -0-20
K 11		ROADWAY		i FIRM / LC	DGGER: ODOT/	MCLEISH			CME	AUTO	MATIO	;	ALIG	NME	NI: _			SR 52	29	L		PAGE
	PID: <u>112818</u>	_ SFN:		METHOD:	2.25" HS	A		LIBRAI	ION DATE:		4/15/2	0		/ATIO	N:	978.	0 (MS	iL)	EOB:	7.5 ft.		1 05 1
	START: <u>3/8/21</u>	END: <u>3/8/21</u>	SAMPLING	METHOD	: SPT		EN	ERGY F	ratio (%):		83.6		LAT	/ LON	G:		40	.54142	27, -83.	.010703		TOFT
	MATERIA	L DESCRIPTION		ELEV.	DEDTUS	SPT/	NI	REC	SAMPLE	HP		GRAD	OITA	ا (%)		ATT	ERBE	RG		ODOT	SO4	BACK
	AN	ID NOTES		978.0	DEPINS	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
1.0' - AS	PHALT (12.0")			977.0	1 _	_																
0.5' - AG	GREGATE BASE (6.0")		976.5																		SUM S
VERY ST COARSE	TIFF, DARK GRAY E TO FINE SAND, T	SILT AND CLAY , TRACE RACE FINE GRAVEL, MO	IST.		- 2 - - 3 -	6 4 4	11	83	SS-1	3.00	3	4	6	41	46	34	21	13	19	A-6a (9)	140	
				973.5	- 4 -	-2 3 3	8	0	AS-2	-	-	-	-	-	-	-	-	-	19	A-6a (V)	-	
SOFT TO TRACE (D MEDIUM STIFF, E COARSE TO FINE \$	BROWN CLAY , "AND" SILT SAND, MOIST.	r,		- 5 -	2 2 3	7	100	SS-3	1.00	0	2	9	38	51	51	20	31	27	A-7-6 (18)	-	
				970.5	EOB	2 ² 22	6	100	SS-4	0.50	-	-	-	-	-	-	-	-	26	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529- ROADW	2.59 FDR AY	DRILLING	FIRM / OPE G FIRM / LO	RATOR: _	ODOT / ODOT / N	CAREY ICLEISH	DRI HAI	ILL RIG	: CM CME	E 55 T AUTO	RUCK MATIC		STA ALIG		/ OFF	SET:		SR 52	' !9	E	XPLOR B-050	ATION ID - 0-20
	PID: 112818	SFN:			METHOD:	2	2.25" HSA		CAL	IBRAT	ION DATE:		4/15/2	0	ELE	VATIC	N:	979.0) (MSL	_)	EOB:	7.5 ft.		PAGE
	START: <u>3/2/</u>	21 END:	3/2/21	SAMPLIN	3 METHOD:		SPT		EN	ERGY I	RATIO (%):		83.6		LAI	/ LON	G:		40.5	54100)4, -83.	009365		
	MATER	IAL DESCRI	PTION		ELEV.	DEPT	нs	SPT/	Naa	REC	SAMPLE	ΗP	Ģ	SRAD		<u>N (%)</u>		ATTE	RBEF	RG		ODOT	SO4	BACK
	/	ND NOTES			979.0			RQD	• •60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASI	PHALT (12.0")				978.0			-																
0.5' - AG	GREGATE (MAC	ADAM) BASE	Ξ (6.0")		977.5		- 1 -	1																Same of
MEDIUM	I DENSE, GRAY (RAVEL WIT	TH SAND, WET.		976.0		- 2 -	6 4 4	11	-	SS-1	-	-	-	-	-	-	-	-	-	16	A-1-b (V)	100	
MEDIUM SAND, S	I DENSE, BROWI SILT, AND CLAY,	NISH GRAY DAMP.	GRAVEL WITH		974.5		- 3 -	. ³ 4 7	15	33	SS-2	-	-	-	-	-	-	-	-	-	8	A-2-6 (V)	-	
VERY ST SOME C DAMP TO	TIFF, BROWN SIL COARSE TO FINE O MOIST.	T AND CLA SAND, TRA	Y , LITTLE TO CE FINE GRAVE	∃L,			- 5 -	4 5 6	15	100	SS-3	4.00	5	17	9	35	34	33	18	15	20	A-6a (9)	-	
					971.5	—FOB—	- 7 -	4 7 10	24	100	SS-4	4.00	9	8	11	36	36	33	18	15	14	A-6a (9)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: <u>MAR-529-2.59 FDR</u> DRILLII TYPE: <u>ROADWAY</u> SAMPL PID: 112818 SFN: DRILLII	NG FIRM / OPI ING FIRM / LC NG METHOD:	ERATOR: ODOT / CAREY OGGER: ODOT / MCLEISH 2.25" HSA	DF HA CA	RILL RIG MMER: LIBRAT	::CM CME ION DATE:	E 55 T AUTO	RUCK MATIC 4/15/2	: C O	STA ALIC ELE	TION / SNMEN	/ OFF NT: _)N:	SET:	.0 (MS	SR 52 SL)	' 29 EOB:	E	XPLOR/ B-053	ATION ID -0-20 PAGE
START: <u>3/8/21</u> END: <u>3/8/21</u> SAMPL	ING METHOD	SPT	EN	IERGY I	RATIO (%):		83.6		LAT	/ LON	IG:		40	, .5396	81, -83	.005545		1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 977.0	DEPTHS SPT/ RQD	N ₆₀	REC	SAMPLE	HP (tsf)	GR	GRAD	ATIO FS	N (%) si	CL		ERBE PL	ERG	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASPHALT (12.0") 0.5' - AGGREGATE BASE (6.0")	976.0 975.5																	
MEDIUM STIFF, BROWNISH GRAY SILT AND CLAY , TRACE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	974.0	-2 - 8 - 5 - 3	11	94	SS-1	1.00	9	2	5	36	48	33	18	15	23	A-6a (10)	<100	
STIFF, BROWNISH GRAY CLAY , "AND" SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	972.5	-333345 - 5 - 53 - 53 - 533	13	100	SS-2	1.50	3	4	11	36	46	41	18	23	19	A-7-6 (13)	-	
VERY STIFF, BROWNISH GRAY TO DARK GRAY SANDY SILT, LITTLE CLAY, TRACE FINE GRAVEL, MOIST.			21	83	SS-3	3.00	-	-	-	-	-	-	-	-	16	A-4a (V)	-	
	969.5	EOB	33	100	SS-4	3.50	-	-	-	-	-	-	-	-	16	A-4a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE: PID: 112818	MAR-529-2. ROADWA SFN:	.59 FDR \Y	DRILLING SAMPLING DRILLING	FIRM / OPE 3 FIRM / LO METHOD:	ERATOR:	ODOT / ODOT / M 2.25" HSA	CAREY CLEISH	DRI HAM CAL	LL RIG MMER: _IBRAT	:CM CME ION DATE:	IE 55 T AUTO	RUCK MATI(4/15/2	: C O	STAT ALIGI ELEV	ION / C IMENT ATION	0FFSET : 976	: 6.0 (M:	<u>SR 5</u> ; SL)	' 29 EOB:	E	XPLOR/ B-054	TION ID -0-20 PAGE
	START: 3/2/2	21 END:	3/2/21	SAMPLING	3 METHOD	:	SPT		ENE	ERGY F	RATIO (%):		83.6		LAT /	LONG		40).5392	<u>55, -83</u>	.004209		1 OF 1
	MATERI A	AL DESCRIP	TION		ELEV. 976.0	DEP	THS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	GRAD/ CS	ATION FS	(%) si c	AT		ERG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - AS 0.5' - AG	BPHALT (12.0") BGREGATE (MACA	ADAM) BASE	(6.0")		975.0																		324
Medium And Sil	I DENSE, DARK G .T, TRACE CLAY,	RAY GRAVE WET.	L WITH SAND		973.0		- 2 -	7 4 4	11	17	SS-1	-	-	-	-		-	-	-	24	A-2-4 (V)	<100	
MEDIUM BROWN TO FINE	A STIFF TO STIFF IISH GRAY SILT A SAND, TRACE FI	DARK GRAY I ND CLAY , LI INE GRAVEL,	TO TTLE COARSE , DAMP.				- 4 -	2 3 5	11	17	SS-2	1.00	-	-	-		-	-	-	11	A-6a (V)	-	
					970.0		- 5 -	4 4 5	13	89	SS-3	2.00	9	6	8	35 42	2 32	19	13	15	A-6a (9)	-	
VERY S COARSE	TIFF, BROWNISH E TO FINE SAND,	GRAY SILTY TRACE FINE	CLAY, LITTLE GRAVEL, MOI	ST.	968.5	—FOB—	- 7 -	4 6 9	21	100	SS-4	3.50	10	7	11 ;	37 3	5 37	19	18	19	A-6b (11)	-	

02019 RII STAND ODOT LOG SULF (8.5 X 11) - OH DOT.GDT - 4/9/21 14:19 - U:\GI8\PROJECTS\2020\W-20-161.GPJ

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii) P	PROJECT:	MAR-529-2.59 FDR ROADWAY	DRILLING SAMPLING	FIRM / OPE 6 FIRM / LO	RATOR:(GGER:OE	ОДОТ / С ООТ / МС	AREY	DRI HAI	ILL RIG MMER:	: <u>CM</u> CME	E 55 T AUTO	RUCK MATIC))	STA ALIG	TION /	OFFS	ET: _	S	SR 52	, 9	E	XPLOR B-057	ATION ID 2-0-20
P	PID: <u>112818</u>	_ SFN:	DRILLING	METHOD:	2.2	25" HSA		CAL	LIBRAT	ION DATE:		4/15/2	0	ELE	VATIO	N:	980.0	(MSL	.)	EOB:	7.5 ft.		PAGE
S	START: <u>3/8/2</u>	<u>1 </u>	SAMPLING	METHOD:		SPT		EN	ERGY I	KATIO (%):		83.6		LAI	/LON	G:		40.5	53788	31, -83.	000279		1011
	MATERIA	AL DESCRIPTION		ELEV.	DEPTH	s	SPT/	N.,	REC	SAMPLE	ΗP	Ċ	RAD		<u>N (%)</u>	/	ATTE	RBEF	RG		ODOT	SO4	BACK
	Al	ND NOTES		980.0		0	RQD	• 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASPH/	ALT (12.0")	(6.0")		979.0	-	- 1																	
STIFF, DAR LITTLE COA MOIST.	RK BROWNISH ARSE TO FINE	GRAY SILT AND CLAY , SAND, TRACE FINE GRAV	EL,	977.0	-	- 2 - 6	6 6 4	14	0	AS-1	-	5	8	10	45	32	38	23	15	19	A-6a (10)	140	
STIFF, BRO SILT, LITTLI GRAVEL, M	OWN TO BROW E COARSE TO MOIST.	NISH GRAY CLAY , "AND" FINE SAND, TRACE FINE			-	- 4 -	5 4 5	13	0	AS-2	-	5	4	7	37	47	52	19	33	23	A-7-6 (18)	-	
				974.0	-	- 5 4	4 4 3	10	56	SS-3	2.00	-	-	-	-	-	-	-	-	28	A-7-6 (V)	-	
VERY STIFF LITTLE COA MOIST.	F, BROWNISH (ARSE TO FINE S	GRAY SILT AND CLAY , SAND, TRACE FINE GRAV	EL,	972.5	ЕОВ	- 7 -	⁴ 4 3	10	89	SS-4	3.00	-	-	-	-	-	-	-	-	17	A-6a (V)	-	

02019 RII STAND ODOT LOG SULF (8.5 X 11) - OH DOT.GDT - 4/9/21 14:19 - U:\GI8\PROJECTS\2020\W-20-161.GPJ

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: <u>MAR-529-2.59 FDR</u> DR	rilling fi Ampling i	IRM / OPE FIRM / LO	ERATOR: <u>ODOT</u> GGER: <u>ODOT</u>	/ CAREY MCLEISH	DRI HAI	ILL RIG MMER:	: CM CME	E 55 TI AUTOI	RUCK		STA ALIG	TION SNME	/ OFF NT: _	SET:		SR 52	, 29	E	XPLOR B-058	ATION ID 3-0-20
PID: <u>112818</u> SFN: DR START: <u>3/2/21</u> END: <u>3/2/21</u> SA	rilling m Ampling i	IETHOD: METHOD	2.25" HS :SPT	SA		librat Ergy f	ION DATE: RATIO (%):		4/15/2 83.6	0	ELE ^V LAT	/ATIC / LON	DN: _ \G: _	980.	0 (MS 40	SL) .5374	EOB: 58, -82	7.5 ft. .998939		1 OF 1
MATERIAL DESCRIPTION AND NOTES		ELEV. 980.0	DEPTHS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	GRAD/ CS	ATIOI FS	N (%) SI) CL	ATT	ERBE PL	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.8' - ASPHALT (10.0") 0.4' - AGGREGATE (MACADAM) BASE (4.0")		979.2 978.8		-																
VERY STIFF, DARK GRAY SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.		977.0	- 2		11	56	SS-1	3.00	8	7	5	46	34	38	23	15	17	A-6a (10)	160	
MEDIUM STIFF TO STIFF, DARK GRAY TO BROWNISH GRAY CLAY , SOME SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	r.		4	2 3 4	10	67	SS-2	1.00	15	3	8	32	42	50	21	29	25	A-7-6 (17)	-	
			- 5	-2 3 4	10	100	SS-3	1.50	0	3	12	41	44	47	19	28	26	A-7-6 (17)	-	
		972.5	СОВСО	- ³ 3 4	10	56	SS-4	1.50	-	-	-	-	-	-	-	-	20	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRIL TYPE: ROADWAY SAM PID: 112818 SFN: DRIL	LING FIF IPLING FI LING ME	RM / OPER IRM / LOG ETHOD:	ATOR: <u>ODOT /</u> GER: <u>ODOT / M</u> 2.25" HSA	CAREY CLEISH	DRI HAI CAI	ILL RIG: MMER: LIBRAT	:CM CME ION DATE:	E 55 T AUTO	RUCK MATIC 4/15/2))	STA ALIG ELE	TION SNME VATIO	/ OFF NT: _ DN: _	SET: 976.	0 (MS	SR 52 L)	, 29 EOB:	E	XPLOR B-061	ATION ID -0-20 PAGE
START: <u>3/8/21</u> END: <u>3/8/21</u> SAM	IPLING M	IETHOD: <u></u>	SPT	SPT/	ENE	ERGY F	RATIO (%): SAMPLE	HP	83.6 C	GRAD/	LAT ATIOI	/ LON N (%)	NG:)	ATT	40. ERBE	53608 RG	33, -82.	.994998 ODOT	S04	1 OF 1 BACK
AND NOTES		976.0	DEPTHS	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
1.0' - ASPHALT (12.0") 0.5' - AGGREGATE BASE (6.0")		975.0						<u>,</u>												
STIFF, BROWN SANDY SILT , SOME CLAY, TRACE FINE GRAVEL, DAMP.		973.0	- 2 -	3 3 3	8	78	SS-1	1.50	3	15	13	37	32	29	19	10	17	A-4a (7)	350	
VERY STIFF TO HARD, BROWNISH GRAY TO BROWN SANDY SILT, SOME CLAY, TRACE FINE GRAVEL, DAMP.			- 4 -	4 6 12	25	78	SS-2	3.00	7	10	13	40	30	27	17	10	13	A-4a (7)	-	
			- 5 -	10 10 10	28	83	SS-3	4.50	-	-	-	-	-	-	-	-	13	A-4a (V)	-	
		968.5	- 0 - - 7 -	9 10 14	33	100	SS-4	4.50	-	-	-	-	-	-	-	-	13	A-4a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

D	PROJECT:	MAR-529-2.59) FDR	DRILLING	FIRM / OP	ERATOR:		CAREY		ILL RIG	:CM	E 55 T	RUCK	2	STA		/ OFF	SET:		SR 52	' 29	E	EXPLOR	ATION ID 2-0-20
	PID: <u>112818</u> START: 3/1/2	SFN: 1END:	3/1/21	DRILLING	METHOD: METHOD): 	2.25" HSA SPT			LIBRAT ERGY F	ION DATE: RATIO (%):		4/15/2 83.6	0	ELE	VATIC / LON	DN: _ IG:	975.	0 (MS 40	<u>L)</u> 53570	EOB: 2, -82	7.5 ft 7.5 ft		PAGE 1 OF 1
	MATERIJ	AL DESCRIPTI ND NOTES	ON		ELEV. 975.0	DEPT	ΉS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE	HP (tsf)	GR	BRAD cs	ATIO FS	N (%) si) CL		ERBE PL	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.1' - ASI	PHALT (13.0")	DAM) BASE (F	5.0")		973.9 973.5			-																
VERY ST SOME C DAMP.	ORECATE (MACA FIFF, BROWNISH (OARSE TO FINE S	GRAY SILT AN SAND, LITTLE I	ID CLAY, FINE GRAVE	L,	972.0		- 2 -	5 6 4	14	56	SS-1	3.00	12	15	9	28	36	34	19	15	13	A-6a (8)	360	
stiff, B Brown Sand, T	ROWNISH GRAY CLAY, "AND" SILT RACE FINE GRAV	TO GRAY AND T, LITTLE COAI ′EL, DAMP TO) grayish RSE to fine Moist.				- 3 - - 4 -	1 2 3	7	67	SS-2	2.00	1	3	11	48	37	41	22	19	28	A-7-6 (12)) -	
							- 5 -	3 4 5	13	67	SS-3	2.00	-	-	-	-	-	-	-	-	29	A-7-6 (V)	-	
					967.5	EOB-	- 7 -	3 2 3	7	100	SS-4	2.00	-	-	-	-	-	-	-	-	24	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529-2.59 FDR ROADWAY	DRILLING SAMPLIN(FIRM / OPE G FIRM / LO [,]	RATOR: _	ODOT / /	CAREY CLEISH	DRI HAI	ILL RIG MMER:	: CM	E 55 T AUTO	RUCK MATIO	: C	STA [®] ALIG		OFF	ET:		SR 52	' 29	E	XPLOR/ B-065	ATION ID - 0-20
	PID: 112818	_ SFN:		METHOD:	2	2.25" HSA		CAL	IBRAT	ION DATE:		4/15/2	0	ELE	VATIO	N:	986.0) (MSI	_)	EOB:	7.5 ft.		PAGE
	START: 3/8/2	1 END: <u>3/8/21</u>	SAMPLING	3 METHOD:		SPT		ENE	ERGY	RATIO (%):		83.6		LAT	/LON	G:		40.	53435	56, -82	.989986		TOFT
	MATERIA	AL DESCRIPTION		ELEV.	DEPT	нs	SPT/	N.,	REC	SAMPLE	ΗP	0	GRAD	ATIO	N (%)			RBE	RG		ODOT	SO4	BACK
	AI	VD NOTES		986.0			RQD	• •60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASI	PHALT (12.0")			985.0																			
0.5' - AG	GREGATE BASE	(6.0")		984.5		רי ק																	Serve S
STIFF, G SAND, TI	GRAY SILTY CLAY , RACE FINE GRAV	LITTLE COARSE TO FINI EL, DAMP TO MOIST.	=	983.0		- 2 -	5 4 4	11	61	SS-1	2.00	10	9	11	35	35	35	19	16	25	A-6b (9)	320	
VERY ST COARSE	TIFF TO HARD, BR E TO FINE SAND, T	OWN SILTY CLAY , LITTL RACE FINE GRAVEL, DA	т МР.			- 4 -	4 4 6	14	50	SS-2	2.50	-	-	-	-	-	-	-	-	17	A-6b (V)	-	
						- 5 -	7 8 10	25	78	SS-3	4.00	8	9	12	35	36	35	18	17	14	A-6b (10)	-	
-LARGE	E GRAVEL FRAGM	IENT IN SS-4		978.5	—FOB—	- 7 -	7 9 12	29	100	SS-4	4.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

		MAR-529-2.59 FDR		FIRM / OP							E 55 T	RUCK	< <u> </u>	STA		/ OFF	SET:		SD 5'	' 20	E	XPLOR B-066	ATION ID 5 -0-20
RI	PID: <u>112818</u>	SFN:		METHOD:	2.	.25" HSA				ION DATE:		4/15/2	0	ELE		DN: _	987.	.0 (MS	SL)	EOB:	7 .5 ft.		PAGE
	START: 3/1/21	END: <u>3/1/21</u>	SAMPLIN	G METHOD	:	SPT		ENE	ERGY F	RATIO (%):		83.6		LAT	/ LON	NG: _		40	.5339	26, -82	.988639		1 OF 1
	MATERIA	L DESCRIPTION		ELEV.		JC	SPT/	N	REC	SAMPLE	HP	0	GRAD	OATIO	N (%)	ATT	ERBE	ERG		ODOT	SO4	BACK
	AN	ND NOTES		987.0	DEPTE	10	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
0.8' - AS	PHALT (9.5") ND BASE (3.0")			986.2 985.9																			
STIFF, G SAND, T	GRAY SILTY CLAY , RACE FINE GRAVI	LITTLE COARSE TO FIN EL, MOIST.	IE	984.0		- 2	3 6 5	15	1	SS-1	2.00	6	4	8	40	42	35	19	16	22	A-6b (10)	300	
STIFF TO TRACE (DAMP TO	O VERY STIFF, GR COARSE TO FINE \$ O MOIST.	AY CLAY , "AND" SILT, SAND, TRACE FINE GR/	VEL,			_ 3 _ _ 4 _	3 3 6	13	83	SS-2	1.50	2	2	5	37	54	58	21	37	25	A-7-6 (20)	-	
						- 5	3 4 6	14	89	SS-3	2.50	-	-	-	-	-	-	-	-	21	A-7-6 (V)	-	
				979.5	EOB-	 7	4 5 6	15	89	SS-4	3.00	-	-	-	-	-	-	-	-	19	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: <u>MAR-529-2.59 FDR</u> DRILLIN TYPE: <u>ROADWAY</u> SAMPLII	3 FIRM / OP IG FIRM / L(ERATOR: ODOT / CAREY	DR HA	ILL RIG MMER:	:CM	E 55 T AUTO	RUCK MATI(: C	STA ALIC	TION SNME	/ OFF	SET:		SR 52	' 29	E	XPLOR B-06	ATION ID 9-0-20
PID: <u>112818</u> SFN: DRILLIN' START: <u>3/9/21</u> END: <u>3/9/21</u> SAMPLI	3 METHOD:	2.25" HSA		LIBRAT	ION DATE:		4/15/2 83.6	0	ELE'		2N: _	989.	0 (MS.	SL)	EOB:	7.5 ft.		PAGE 1 OF 1
MATERIAL DESCRIPTION	ELEV.	DEPTHS SPT/	N	REC	SAMPLE	HP	00.0	GRAD	ATIO	N (%))	ATT	ERBE	RG	10, -02	ODOT	S04	BACK
AND NOTES	989.0	RQD	• •60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
0.8' - ASPHALT (10.0")	988.2																	
0.7' - AGGREGATE BASE (8.0")	987.5																	
SOFT, DARK GRAYISH BROWN SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	986.0	-2 -3 2 -3 2 -3 2 -3 -3 -3 -3 -3 -3 -3 -3	6	89	SS-1	0.50	5	6	8	54	27	34	19	15	22	A-6a (10)	200	
SOFT TO VERY STIFF, BROWN CLAY , "AND" SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3	8	89	SS-2	0.50	1	4	8	51	36	51	19	32	25	A-7-6 (18)	-	
		-5 -2	8	83	SS-3	1.00	-	-	-	-	-	-	-	-	24	A-7-6 (V)	-	
	981.5	EOB	13	100	SS-4	2.50	-	-	-	-	-	-	-	_	18	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJEC ⁻ TYPE:	Г:	MAR-529-2.5 ROADWAY	59 FDR	DRILLING SAMPLING	FIRM / OP G FIRM / LO	ERATOR:		CAREY	DRI HAI	ILL RIG MMER:	:CM	E 55 T AUTO	RUCK MATIK	()	STAT		/ OFF NT:	SET:		SR 52	' 29	E	EXPLOR	ATION ID)-0-20
	PID: 1	12818	SFN:		DRILLING	METHOD:	_	2.25" HSA	1	CAL	LIBRAT	ION DATE:	4	4/15/2	0	ELEV	ATIC)N:	987.	0 (MS	L)	EOB:	7.5 ft		PAGE
	START:	3/1/21	END:	3/1/21	SAMPLING):	SPT		EN	ERGY F	RATIO (%):		83.6		LAT /	LON	IG:		40.	53215	55, -82.	.983338		1 OF 1
	Ι	MATERIAL	DESCRIPT	TION		ELEV.	DEPT	гнs	SPT/	N	REC	SAMPLE	HP	0	GRAD	ATION	l (%)		ATT	ERBE	RG		ODOT	SO4	BACK
		ANE	D NOTES			987.0			RQD	• •60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
0.9' - ASF	PHALT (11	.0")		(2.0")		986.1		1 -	-																
			AIVI) BASE			305.0																			Pg L alt
LITTLE F DAMP. -TRACE	INE GRA	/EL, TRAC	RAY SILTA E COARSE 1	TO FINE SAN	JD,	984.0		- 2 -	-6 4 5	13	78	SS-1	2.00	12	4	5	49	30	35	22	13	21	A-6a (9)	240	
SOFT TC SOME SI FINE GR) STIFF, B ILT, LITTLI AVEL, MC	ROWNISH E COARSE DIST.	I DARK GR E TO FINE \$	AY CLAY , SAND, TRACE				- 3 - - 4 -	-3 -3 -4	10	61	SS-2	1.50	1	3	7	54	35	42	19	23	25	A-7-6 (14)	-	
								- 5 -	2 2 3	7	44	SS-3	0.50	-	-	-	-	-	-	-	-	21	A-7-6 (V)	-	
						979.5	EOB-	- 6 - - 7 -	2 2 3	7	50	SS-4	0.50	-	-	-	-	-	-	-	-	22	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529-2.59 FDR ROADWAY	DRILLING SAMPLIN	FIRM / OP	ERATOR: <u>C</u> DGGER: OD	DOT / CARE	Y DF	MMER:	: CN CME	IE 55 T AUTO	RUCK		STA		/ OFF NT:	SET:		SR 5	29	E	XPLOR B-073	ATION ID 3 -0-20
	PID: 112818	SFN:		METHOD:	2.25	5" HSA	CA	LIBRAT	ION DATE:		4/15/2	0	ELE	VATIC	DN: _	990.	.0 (MS	SL)	EOB:	7.5 ft.		PAGE
	START: <u>3/9/21</u>	END: 3/9/21	SAMPLING	3 METHOD	D:	SPT	EN	IERGY I	Ratio (%):		83.6		LAT	/LON	IG:		40	.5307	89, -82	.979389		1011
	MATERIA	L DESCRIPTION		ELEV.	DEPTHS	SP1	7 N	REC	SAMPLE	HP	9	GRAD	OITA	N (%))	ATT	ERBE	ERG		ODOT	SO4	BACK
	AN	D NOTES		990.0	DEITIG	RQI	D	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
0.8' - AS	PHALT (10.0")			989.2	_	_																
0.7' - AG	GREGATE BASE (8	3.0")		988.5		- 1 -																
STIFF, G FINE SA	GRAY SILT AND CL A ND, LITTLE FINE G	AY, LITTLE COARSE T RAVEL, DAMP.	ГО	987.0	-	- 2 - ⁵ 4	4	56	SS-1	2.00	16	5	7	32	40	35	23	12	18	A-6a (8)	<100	
MEDIUM "AND" SI FINE GR	N STIFF TO STIFF, C ILT, LITTLE COARS RAVEL, MOIST.	3RAY TO BROWN CL E TO FINE SAND, TR/	AY, ACE		-	- 4 - 4	13 5	100	SS-2	1.50	1	3	8	38	50	49	21	28	24	A-7-6 (17)	-	
					-	- 5 - ² 2	3 7	61	SS-3	1.00	-	-	-	-	-	-	-	-	28	A-7-6 (V)	-	
				982.5	EOB	- 7 - ³	4	100	SS-4	2.00	-	-	-	-	-	-	-	-	22	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRILI TYPE: ROADWAY SAME PID: 112818 SFN: DRILI START: 2/4/21 SAME SAME	LING FIRM / OP PLING FIRM / LC LING METHOD:	ERATOR: ODOT / CAREY OGGER: ODOT / MCLEISH 2.25" HSA	DRILL HAMM CALIE	L RIG: MER: BRATI	CMI CME ON DATE:	E 55 TI AUTOI	RUCK MATIC	; ; ;	STAT ALIGI ELEV	ION / NMEN ATIO	/ OFF NT: N:	SET: 990.	0 (MS	SR 52	' 29 EOB:	E	XPLOR/ B-074	ATION ID -0-20 PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV. 990.0	DEPTHS SPT/ RQD		REC (%)	SAMPLE ID	HP (tsf)	GR	GRAD/ CS	ATION FS	I (%) SI	CL		ERBE PL	RG	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
1.0' - ASPHALT (12.0") VERY STIFF, GRAYISH BROWN SILT AND CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.	989.0		13	89	SS-1	2.50	5	4	8	39	44	35	23	12	21	A-6a (9)	<100	
-TRACE ORGANICS IN SS-1 STIFF TO VERY STIFF, GRAYISH BROWN SILTY CLAY , LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIST.	987.0	$\begin{array}{c} -3 \\ -3 \\ -4 \\ -4 \\ -5 \\ -5 \end{array}$	13	89	SS-2	1.50	5	5	12	40	38	40	18	22	20	A-6b (13)	-	
		-5 -4 - 5 - 6	15 ⁻	100	SS-3	3.00	-	-	-	-	-	-	-	-	22	A-6b (V)	-	
	982.5	= - 5 - 7 - 6 - 7 - 9	21 ·	100	SS-4	2.50	-	-	-	-	-	-	-	-	21	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Dii	PROJECT:	MAR-529-2 ROADWA	.59 FDR		FIRM / OP G FIRM / L(ERATOR:	/ TODO ODOT / N	CAREY		ILL RIG	: CM	E 55 T AUTO	RUCK MATIO	2	STA		/ OFF NT:	SET:		SR 52	29	E	XPLOR B-077	ATION ID - 0-20
	PID: 112818	SFN:	2/0/21		METHOD:	·	2.25" HSA				ION DATE:		4/15/2)	ELE	VATIO	DN: _	994.	.0 (MS	SL)	EOB:	7.5 ft.		PAGE 1 OF 1
	MATE	RIAL DESCRIP	TION		ELEV.		<u>эгт</u>	SPT/		REC	SAMPLE	HP	63.0	RAD		N (%))	ATT	ERBE	.52900 RG	03, -02.	ODOT	SO4	BACK
		AND NOTES			994.0		113	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	ΡI	WC	CLASS (GI)	ppm	FILL
0.9' - ASI	PHALT (10.5")				993.2			-																
0.7' - AG	GREGATE BAS	Ξ (8.0")			992.5		- 1 -																	9 L 4
GRAY S/ GRAVEL	andy silt , soi ., damp.	ME CLAY, LITT	LE FINE		991.0		- 2 -	5 5 4	13	0	AS-1	-	13	10	11	38	28	29	24	5	12	A-4a (6)	<100	
STIFF TO CLAY, "A TRACE F	O VERY STIFF, AND" SILT, LITT FINE GRAVEL, [BROWNISH GF LE COARSE TO DAMP TO MOIS	₹AY TO BROW) FINE SAND, \$T.	N			- 4 -	2 4 4	11	67	SS-2	1.50	10	6	8	38	38	42	19	23	22	A-7-6 (14)	-	
					-		- 5 -	2 4 4	11	94	SS-3	2.00	-	-	-	-	-	-	-	-	22	A-7-6 (V)	-	
					986.5	EOB-	- 7 -	2 4 5	13	100	SS-4	2.50	-	-	-	-	-	-	-	-	17	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR TYPE: ROADWAY PID: 112818 START: 3/1/21	DRILLING SAMPLING DRILLING SAMPLING	FIRM / OP 6 FIRM / LC METHOD: 6 METHOD	ERATOR: <u>ODOT /</u> OGGER: <u>ODOT / M</u> 2.25" HSA : SPT	CAREY	DRI HAI CAI ENI	ILL RIG MMER: LIBRAT ERGY F	:CM CME ION DATE: RATIO (%):	E 55 T AUTO	RUCK MATIC 4/15/2 83.6	2 0	STA ALIO ELE LAT	TION SNME VATIO / LOI	/ OFF :NT: _ ON: _ NG:	SET:	.0 (MS 40	SR 52 iL) .5286;	' <u>29</u> EOB: 37, -82	E 7.5 ft. .972897	XPLOR/ B-078	ATION ID -0-20 PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	-1	ELEV. 995.0	DEPTHS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE	HP (tsf)	GR	GRAD CS	ATIO FS	N (% si) CL	ATT LL	ERBE PL	RG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.8' - ASPHALT (10.0") 0.2' - AGGREGATE BASE (2.0") STIFF, GRAYISH BROWN SANDY SILT , LITTLE FINE GRAVEL, TRACE COARSE TO FINE SAND, DRY. STIFF TO VERY STIFF, BROWNISH GRAY CLAY ,		994.2 994.0 992.0	- 1 - - 2 - - 3 -	10 5 7	17	22	SS-1	-	13	3	6	29	49	29	23	6	6	A-4a (8)	<100	
"AND" SILT, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		987.5	- 4 - - 5 - - 6 - - 7 - EOB	$\begin{array}{c} 4 \\ 5 \\ 3 \\ 4 \\ 3 \\ 4 \\ 7 \end{array}$	13 10 15	78 83 100	SS-2 SS-3 SS-4	2.50 3.00 2.00	5 - -	-	9 - -		-	42 - -	- -	-	26 20 16	A-7-6 (14)	-	AL BANG AN UR X CHAN & MI

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DRILLI TYPE: ROADWAY SAMPL	NG FIRM / OP .ING FIRM / LC	ERATOR: <u>ODOT / CARE</u> DGGER: <u>ODOT / MCLEIS</u>	EY DF	RILL RIG	:CM	E 55 T AUTO	RUCK MATIC))	STA ALIG		/ OFF NT: _	SET:		SR 5	' 29	E	XPLOR B-081	ATION ID -0-20
PID: <u>112818</u> SFN:DRILLI	NG METHOD:	2.25" HSA	CA	LIBRAT	ION DATE:		4/15/2	0	ELE'	VATIO	DN: _	1000	0.0 (M	SL)	EOB:	7.5 ft.		PAGE
START: <u>3/9/21</u> END: <u>3/9/21</u> SAMPL	ING METHOD	: <u>SPT</u>	EN	IERGY F	Ratio (%):		83.6		LAT	/LON	NG: _		40	.5272	88, -82	.969048		1 OF 1
MATERIAL DESCRIPTION	ELEV.	DEDTUS SPT.	7	REC	SAMPLE	HP	0	RAD	ATIO	N (%)	ATT	ERBE	ERG		ODOT	SO4	BACK
AND NOTES	1000.0	RQE	D 1960	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
1.0' - ASPHALT (12.0")	9999.0																	
U.5' - AGGREGATE BASE (6.0")	≪ 998.5		_															A Latt
MEDIUM STIFF TO STIFF, BROWNISH GRAY SILTY CLAY , SOME COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.	997.0	- 2 - ⁸ 5	5 14	61	SS-1	1.00	1	2	21	51	25	36	20	16	16	A-6b (10)	120	
STIFF TO VERY STIFF, BROWN TO BROWNISH GRAY SILT AND CLAY, SOME COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP TO MOIST.		2 2 3 4 3	4 10	100	SS-2	2.00	10	8	16	37	29	32	17	15	27	A-6a (8)	-	
		_ 5 _ 5 _ 3	4 10	89	SS-3	2.00	-	-	-	-	-	-	-	-	17	A-6a (V)	-	
	992.5	EOB	25	89	SS-4	4.00	-	-	-	-	-	-	-	-	20	A-6a (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

D ::	PROJECT: TYPE:	MAR-529-2.59 FDR ROADWAY	DRILLING	FIRM / OP G FIRM / LC	ERATOR:	ODOT / ODOT / MO	CAREY	DRI HAI	ILL RIG MMER:	: <u>CM</u> CME	E 55 T AUTO	RUCK	2	STA		/ OFF	SET:		SR 52	' 29	E	XPLOR B-082	ATION ID 2 -0-20
	PID: 112818	SFN:	DRILLING	METHOD:	2	.25" HSA			IBRAT	ION DATE:		4/15/2	0	ELE	VATIO	- DN: _	999.	0 (MS	SL)	EOB:	7.5 ft.		PAGE
	START:3/1	/21 END: <u>3/1/21</u>	SAMPLING	G METHOD):	SPT		ENE	ERGY F	RATIO (%):		83.6		LAT	/LON	NG: _		40	.5268	61, -82	.967709		1 OF 1
	MATE	RIAL DESCRIPTION		ELEV.			SPT/	N	REC	SAMPLE	ΗP	0	GRAD	ATIO	N (%)	ATT	ERBE	RG		ODOT	SO4	BACK
		AND NOTES		999.0		15	RQD	1 4 60	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
0.8' - ASI	PHALT (10.0")	- (4.00)		998.2																			
0.3' - AG	GREGATE BASE	<u>= (4.0")</u>	<u></u>	997.9		- 1																	SER S
MEDIUM SILT , SC	I STIFF, DARK B DME CLAY, LITTI	Rownish Gray Sandy .e fine gravel, moist.		996.0		- 2 -	2 3 3	8	78	SS-1	1.00	12	12	6	46	24	29	19	10	25	A-4a (7)	140	
MEDIUM CLAY , "A TRACE F	I STIFF TO STIF AND" SILT, TRAC FINE GRAVEL, D	F, DARK BROWNISH GRAY CE COARSE TO FINE SAND, DAMP TO MOIST.				- 3 - - 4 -	2 3 4	10	89	SS-2	1.00	3	3	6	40	48	53	20	33	18	A-7-6 (19)	-	
						- 5 -	³ 33 4	52	11	SS-3	1.50	-	-	-	-	-	-	-	-	16	A-7-6 (V)	-	
				991.5	ЕОВ	- 7 -	4 4 5	13	100	SS-4	2.00	-	-	-	-	-	-	-	-	17	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR DR	RILLING FI AMPLING F	IRM / OPE FIRM / LO	RATOR: _	/ TODO M / TODC	CAREY CLEISH		LL RIG MMER:	: CM	E 55 TI AUTO	RUCK	;	STA	TION SNMEI	/ OFF NT:	SET:		SR 52	' 29	E	XPLOR	ATION ID 5-0-20
PID: DR START: SFN: DR START: SI/9/21 SAI	RILLING M AMPLING I	IETHOD: METHOD:	2	2.25" HSA SPT		CAL	LIBRAT ERGY F	ION DATE: RATIO (%):		4/15/20 83.6)	ELE	VATIC / LON	DN:	1003	5.0 (M 40	SL) .5254	EOB: 95, -82.	7.5 ft. 963761		PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES		ELEV. 1003.0	DEPT	HS	SPT/ RQD	N ₆₀	REC (%)	SAMPLE ID	HP (tsf)	GR	CS	ATIO FS	N (%) si) CL	ATT	ERBE PL	ERG PI	wc	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.8' - ASPHALT (10.0") 0.7' - AGGREGATE BASE (8.0")		1002.2 1001.5																			
STIFF, GRAY SILT AND CLAY , SOME FINE TO COARSE SAND, TRACE FINE GRAVEL, MOIST.		1000.0		- 2 -	5 4 3	10	78	SS-1	2.00	5	11	10	55	19	29	18	11	23	A-6a (8)	170	
STIFF TO VERY STIFF, GRAY TO BROWN SILTY CLAY, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, DAMP.				- 4 -	3 4 6	14	100	SS-2	2.50	-	-	-	-	-	-	-	-	16	A-6b (V)	-	
				- 5 -	3 5 7	17	89	SS-3	2.00	10	7	10	35	38	37	19	18	18	A-6b (11)	-	
		995.5	—EOB—	- 7 -	6 9 14	32	78	SS-4	4.00	-	-	-	-	-	-	-	-	16	A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

PROJECT: MAR-529-2.59 FDR I TYPE: ROADWAY S PID: 112818 SFN: S	DRILLING FIRM / SAMPLING FIRM DRILLING METHO	OPERATOR: / LOGGER: _ DD:	ODOT / CAREY ODOT / MCLEISH 2.25" HSA	DR HA CA	ILL RIG MMER: LIBRAT	CME	E 55 T AUTO	RUCK MATIC 4/15/2	C D	STAT ALIGI ELEV	ION / C NMENT ATION:	FFSET	5.0 (M	SR 53	' 29 EOB:	E	EXPLOR B-086	ATION ID 5-0-20 PAGE 1 OF 1
MATERIAL DESCRIPTION AND NOTES	ELEV 1005	V. DEP	THS SPT/ RQD		REC (%)	SAMPLE ID	HP (tsf)	63.0 GR	GRAD		LONG. I (%) SI CI	AT		ERG PI	WC	ODOT CLASS (GI)	SO4 ppm	BACK FILL
0.9' - ASPHALT (10.5") 0.3' - AGGREGATE (MACADAM) BASE (3.0") HARD, GRAYISH BROWN SILT , SOME CLAY, LITTLE COARSE TO FINE SAND, TRACE FINE GRAVEL, MOIS VERY STIFF, BROWN AND GRAY SILTY CLAY , LITTLE COARSE TO FINE SAND, LITTLE FINE GRAVEL, MOIS	ST	. <u>1</u> .8_ .0	-1 -1 -1 -1 -1 -1 -1 -1	14	89	SS-1 AS-2	4.50	7 7	5	11 8	52 2: 50 3 ⁻	5 29 1 39	19 17	10 22	19 22	A-4b (8) A-6b (13)	180	AN E BAD DARK AN DA T C N C V C V C V C V C V C V C V C V C V
		-		13 14	100 100	SS-3 SS-4	3.00 2.50	-	-	-		-	-	-	19 18	A-6b (V) A-6b (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

Rii	PROJECT: TYPE:	MAR-529-2.59 FDR ROADWAY	DRILLING	FIRM / OPE FIRM / LC	ERATOR: _)GGER: (/ TODO N / TODC	CAREY ICLEISH	DRI HAI	ILL RIG MMER:	: <u>CM</u> CME	IE 55 T AUTO	RUCK MATI(()	STA [®] ALIG	TION SNME	/ OFF NT:	SET:		SR 52	' 29	E	EXPLOR	ATION ID 3-0-20
	PID: 112818	_ SFN:	DRILLING	METHOD:	2	2.25" HSA		CAL	IBRAT	ION DATE:	4	4/15/2	0	ELE	VATIO	DN: _	997.	0 (MS	iL)	EOB:	7.5 ft		PAGE
	START: 3/9/2	1 END: <u>3/9/21</u>	SAMPLING	6 METHOD	:	SPT		ENE	ERGY F	Ratio (%):		83.6		LAT	/LON	NG: _		40	.5237′	13, -82	958680		1 OF 1
	MATERIA	AL DESCRIPTION		ELEV.	DEDT	Пe	SPT/	N	REC	SAMPLE	ΗP	(GRAD	OITA	N (%)	ATT	ERBE	RG		ODOT	SO4	BACK
	Al	ND NOTES		997.0	DEFI	по	RQD	IN ₆₀	(%)	ID	(tsf)	GR	CS	FS	SI	CL	LL	PL	PI	WC	CLASS (GI)	ppm	FILL
0.8' - ASF	PHALT (10.0")			996.2			_																
0.7 - AG	GREGATE BASE	(8.0")		995.5		_ 1 -	1																Salar S
VERY ST LITTLE F MOIST.	TIFF, GRAYISH BR FINE GRAVEL, TRA	OWN SILT AND CLAY , ACE COARSE TO FINE SAI	ND,	994.0		- 2 -	3 4 5	13	89	SS-1	2.50	18	3	6	35	38	30	18	12	22	A-6a (8)	200	
STIFF TC CLAY , SO TRACE F	D HARD, GRAYISH OME SILT, LITTLE FINE GRAVEL, DAI	H BROWN TO BROWN COARSE TO FINE SAND, MP TO MOIST.				- 3 - - 4 -	-3 4 7	15	100	SS-2	2.00	6	5	8	34	47	47	20	27	21	A-7-6 (16)	-	
						- 5 -	6 8 9	24	100	SS-3	4.5+	-	-	-	-	-	-	-	-	15	A-7-6 (V)	-	
				989.5	—EOB—	- 0 - - 7 -	6 9 9	25	100	SS-4	4.50	-	-	-	-	-	-	-	-	15	A-7-6 (V)	-	

NOTES: GROUNDWATER NOT ENCOUNTERED DURING DRILLING

APPENDIX IV

DCP LOGS


NOTES: The latitude, longitude, and elevation values are from a Trimble Geo7X handheld GPS. *Due to method of advancement, surface material values are approximate.



NOTES: The latitude, longitude, and elevation values are from a Trimble Geo7X handheld GPS. *Due to method of advancement, surface material values are approximate. Moved to edge of pavement due to shallow refusal in lane.



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APPENDIX V

Pavement Core Data Sheet

								Pavement Core Data Summary
RESOURCE	6350 Col Telep Fax Nu	Pres umbu hone umbe	ider us, (:: (6 er: (6	ntia Ohio 14) 614	l Ga o 43 823) 82	tewa 231 3-494 3-499	y 9 90	PROJECTMAR-529-2.59LOCATION
	Core Co	mposi	tion					Comments/Remarks
Core Number B-002-0-20	Lift Thickness (in.) 1.75 1.25 1.50 1.00 1.75 1.00 2.00 2.00 4.00	Image: state Image: state ✓ ✓ ✓ ✓ ✓ ✓ ✓ ✓		Concrete	<	Oth		 Core is broken @ 3.0" and 9.0". Break at 3.0" is smooth and along a lift surface. Break along 9.0" lift is degraded and irregular. Bottom of core is irregular from large rocks being embedded within the bottom of the core.
Total Pavement Thickness =	12.25	in.	T -	otal Thicl	Aspł kness	nalt 5 =	12.25	Total Concrete 5 in. Total Concrete 5 in. Thickness = 0.00 in. Thickness = 4.00 in.
						W-20- MAR- B-2		

				Pave	ement Core Da	ta Summary	
RESOURCE	6350 Pre Colum Telephor Fax Numb	esidential G ibus, Ohio 4 ne: (614) 82 ber: (614) 8	ateway 13231 23-4949 23-4990	PROJECT <u>MA</u> LOCATION <u>0</u> JOB No. <u>W-2</u> BORING/CORE No. DATE CORE OBTAI CORE OBTAINED E	R-529-2.59 20-161 INED <u>3/9/2(</u> 3Y <u>ODO</u>	-0-20 021 7 / CAREY + MCLEISH	
	Core Compo	osition			Comments/Re	marks	
Core Number B-017-0-20	Lift Thickness (in.) 0 1.50 √ 0.50 1.00 √ 1.00 √ 1.50 √	 	Other	 Core is broken @ (There are a lot of v broken piece out o Bitumen rich from ; Bottom lift is broke Strong aromatic hy 	3.75', with irregular l oids in the top surfa f the side of the cord 3.75-4.25". n in half and not full odrocarbon odor to s	oreak. ace lift and there is a e from 5.0-6.75". y recovered. ample.	
Total Pavement Thickness =	8.00 in.	Total As Thickne	phalt 8.00	Total Concre in. Thickness =	te 0.00 in. -	Total Base 10.50 Thickness =	in.
			W-20-161 MAR-529-2.59 B-17				

									Pavement Core Data Summary
RESOURCE	6350 Col Telep Fax N	Pres umbu hone umbe	ider us, (e: (6 er: (6	ntia Ohi 14) 514	l Ga o 4 82) 82	atewa 3231 3-494 23-49	ay 49 990		PROJECTMAR-529-2.59LOCATION0JOB No.W-20-161BORING/CORE No.B-022-0-20DATE CORE OBTAINED3/9/2021CORE OBTAINED BYODOT / CAREY + MCLEISH
	Core Co	ompos	ition						Comments/Remarks
Core Number B-022-0-20	Lift Thickness (in.) 1.25 1.75 1.00 1.50 0.75 2.00 1.00 0.75 8.00	Idex Idex Intermediate Intermediate	nalt Base Dase Dase	Concrete	 ≺ Aggregate/Granular Base 		her		 Core broken @ 3.0". Voids, large and small, present throughout core. Highly degraded @ 3.0". Bottom two lifts are bitumen-rich. Bottom of last lift is irregularly shaped on the bottom from embedded large aggregate.
Total Pavement Thickness =	10.00	in.	1	otal Thic	Asp knes	ohalt SS =	10.0	00	in. Total Concrete 0.00 in. Total Base 8.00 in. Thickness =
						W-2 MAI B-22	0-161 R-529-3		

									Pavement Core Data Summary
RESOURCE	6350 Col Telep Fax N	Pre lumt hon umt	eside ous, ie: ((oer: (entia Ohi 314] (614	ıl Ga io 4) 82 1) 8:	atew 3231 3-49 23-4	ay 49 990)	PROJECT MAR-529-2.59 LOCATION 0 JOB No. W-20-161 BORING/CORE No. B-037-0-20 DATE CORE OBTAINED 3/9/2021 CORE OBTAINED BY ODOT / CAREY + MCLEISH
	Core C	ompc	ositior						Comments/Remarks
Core Number B-037-0-20	Lift Thickness (in.) 1.25 1.00 3.50 4.25 8.00	Image: Second state	Intermediate		Aggregate/Granular Base				 Core is intact. Top two lifts are in good condition. Third lift (which could be 2 lifts) has a large concentration of voids.
Total Pavement Thickness =	10.00	in.		Tota Thic	I Asr	 ohalt ss =	10).00	I Total Concrete 0.00 in. Total Base 8.00 in. Thickness = 0.00 in. Thickness =
						W-2 MAI B-37	0-161 R-529- 7	2.59	

										Pavement	Core Data Summary
RESOURCE	6350 Co Telep Fax N	Pres lumb hon umb	side ous, e: (6 er: (ntia Ohi 314) 614	ıl G io 4) 82 1) 8	atew 3231 23-49 23-49	ay 49 990		PROJECT LOCATION JOB No. BORING/COR DATE CORE (CORE OBTAIL	MAR-529-2 0 W-20-161 E No. OBTAINED NED BY	8.59 B-042-0-20 <u>3/9/2021</u> ODOT / CAREY + MCLEISH
	Core Co	ompo	sition	. <u></u> ı						Con	mments/Remarks
Core Number B-042-0-20	Lift Thickness (in.) 1.50 1.75 1.00 1.00 1.50 1.75 2.75 6.00	Ast Surface	>halt Intermediate Base	Concrete	Aggregate/Granular Base				- Core is brok - Core lifts are - Voids are pr - Some large	en, however, it e hard to detern esnet @ 7.5-8.0 aggregate are o	appears it occurred after sitting. nine. .0" and 10.0-10.5" embedded in bottom of asphalt.
Total Pavement Thickness =	11.25	in.		Tota Thic	l As kne	phalt ss =	11.	25	in. Total C	Concrete 0.0 ness =	00 in. Total Base 6.00 in. Thickness =
						W-2 MA B-4	20-161 R-529-2 2	2.59			

								Pavement Core Data Summary
RESOURCE	6350 Co Telep Fax N	Pres lumb ohon umb	side ous, (e: (6 er: ((ntial Ohio 14) 614)	Ga o 43 823) 82	atewa 3231 3-494 23-49	iy 19 90	PROJECTMAR-529-2.59LOCATION0JOB No.W-20-161BORING/CORE No.B-057-0-20DATE CORE OBTAINED3/9/2021CORE OBTAINED BYODOT / CAREY + MCLEISH
	Core Co	ompo	sition					Comments/Remarks
Core Number	Lift Thickness (in.) 1.50	Surface	Base	Concrete	Aggregate/Granular Base	Oth	ner	 Three pieces are partially intact that are 2.75", 2.5", and 2.5" in lengths. Core is highly broken and degraded. Lift thicknesses are not able to be determined.
B-057-0-20	1.25 6.00				✓ 			
Total Pavement Thickness =	2.75	in.	-	Total Thicł	Asp knes	bhalt ss =	2.75	5 in. Total Concrete 0.00 in. Total Base 6.00 in. Thickness = 0.00 in. Thickness =
					W-2 MAI B-57	20-161 R-529-2.5	9	

								Pavement Core Data Summary
RESOURCE	6350 Col Telep Fax N	Pres lumb hone umbe	ider us, (e: (6 er: (6	ntial Dhio 14) 8 514)	Gate 4323 323-4 823-	way 31 1949 499) 0	PROJECTMAR-529-2.59LOCATION0JOB No.W-20-161BORING/CORE No.B-062-0-20DATE CORE OBTAINED3/9/2021CORE OBTAINED BYODOT / CAREY + MCLEISH
	Core Co	ompos	ition					Comments/Remarks
Core Number B-062-0-20	Lift Thickness (in.) 1.75 1.00 1.75 2.00 4.00 5.00	dsy constraints	halt esement e	Concrete				 Core is broken into 4 sections. Core broken @ 2.75", 3.75", 7.5". Core is in bad condition. Layers were presumed from core measurements of below core in photograph. Unknown if order of pieces were correct.
Total Pavement Thickness =	11.50	in.	T	otal / Thick	Asphal ness =	t 1	1.50	I in. Total Concrete 0.00 in. Total Base 5.00 in. Thickness = 5.00 in.
						W-20-1 MAR-52 B-62	61 29-2.59	

										Paveme	nt Co	ore Da	ta Summary	,	
RESOURCE	6350 Col Telep Fax N	Pre lumt hon umb	eside bus, be: (6 ber: (ntia Ohi 614) 614	l Ga o 4: 82) 82	atewa 3231 3-494 23-49	ay 49 990		PROJECT LOCATION JOB No. BORING/COR DATE CORE CORE OBTAI	MAR-52 0 W-20-16 RE No. OBTAINED NED BY	9-2.59 61	B-077 3/9/20 ODOT	-0-20 21 7 / CAREY + M	CLEISF	
	Core Co	ompo	sition								Comm	ents/Rer	narks		
Core Number	Lift Thickness (in.) 1.75 4.00 4.75 8.00	As Surface	Phalt	Concrete	 ≺ Aggregate/Granular Base 		her		- Core is brok	ken @ 5.75". air condition.					
B-077-0-20															
Total Pavement Thickness =	10.50	in.		Total Thic	l Asp knes	ohalt ss =	10.	50	in. Total (Thick	Concrete kness =	0.00	in.	Total Base Thickness =	8.00	in.
						W-20- MAR-5 B-77		59	TICL TARDSTICK NO. 742						

										Pavement	Core Da	ta Summary	1	
RESOURCE	6350 Co Telep Fax N	Pre: lumb)hon umb	side ous, e: (6 er: (ntia Ohi 614	I G o 4) 82 }) 8	atew 3231 23-49 23-4	'ay 49 990		PROJECT LOCATION JOB No. BORING/COR DATE CORE (CORE OBTAIN	<u>MAR-529-2</u> 0 W-20-161 E No. DBTAINED NED BY	8.59 B-082 <u>3/9/20</u> ODOT	-0-20 021 Γ / CAREY + Μ		
	Core Co	ompo	sition							Cor	mments/Rer	marks		
Core Number B-082-0-20	Lift Thickness (in.) 2.00 1.75 2.00 4.25 4.00	Asr Partace	Base Intermediate	Concrete	Aggregate/Granular Base				- Core is broke - Core is in fai - Bottom of the	en @ 5.75". r condition exc e last lift contai	ept along bi	reak.		
Total Pavement Thickness =	10.00	in.		Total Thic	l Ası kne	phalt ss =	10	.00	in. Total C	oncrete ness =	00 in.	Total Base Thickness =	4.00 in.	
							W-200 MAR- B-82	-161 529-3	2.59					

APPENDIX VI

GB1



OHIO DEPARTMENT OF TRANSPORTATION

OFFICE OF GEOTECHNICAL ENGINEERING

PLAN SUBGRADES Geotechnical Bulletin GB1

MARC-529-2.59

112818

Subgrade exploration along SR 529 from SLM 5.29 to SLM 9.33 in Marion County, Ohio.

Resource International Inc.

Prepared By: Peyr Date prepared: Mor

Peyman Majidi, PE Monday, April 5, 2021

Peyman Majidi, PE 6350 Presidential Gateway Columbus, OH 43231 614.823.4949

peymanm@resourceinternational.com

NO. OF BORINGS:

45

5

								Boring	Proposed Subgrado	Cut
#	Boring ID	Alignment	Station	Offset	Dir	Drill Rig	ER	EL.	EL	Fill
1	B-001-0-20	MAR-529-2.59				CME 55	84	975.0	973.5	1.5 C
2	B-002-0-20	MAR-529-2.59				CME 55	84	975.0	973.5	1.5 C
3	B-005-0-20	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
4	B-006-0-20	MAR-529-2.59				CME 55	84	980.0	978.5	1.5 C
5	B-009-0-20	MAR-529-2.59				CME 55	84	977.0	975.5	1.5 C
6	B-010-0-20	MAR-529-2.59				CME 55	84	977.0	975.5	1.5 C
7	B-013-0-20	MAR-529-2.59				CME 55	84	978.0	976.5	1.5 C
8	B-014-0-20	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
9	B-017-0-20	MAR-529-2.59				CME 55	84	982.0	980.5	1.5 C
10	B-018-0-20	MAR-529-2.59				CME 55	84	981.0	979.5	1.5 C
11	B-021-0-020	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
12	B-022-0-20	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
13	B-025-0-20	MAR-529-2.59				CME 55	84	975.0	973.5	1.5 C
14	B-026-0-20	MAR-529-2.59				CME 55	84	972.0	970.5	1.5 C
15	B-029-0-20	MAR-529-2.59				CME 55	84	973.0	971.5	1.5 C
16	B-030-0-20	MAR-529-2.59				CME 55	84	973.0	971.5	1.5 C
17	B-033-0-20	MAR-529-2.59				CME 55	84	973.0	971.5	1.5 C
18	B-034-0-20	MAR-529-2.59				CME 55	84	972.0	970.5	1.5 C
19	B-037-0-20	MAR-529-2.59				CME 55	84	976.0	974.5	1.5 C
20	B-038-0-20	MAR-529-2.59				CME 55	84	976.0	974.5	1.5 C
21	B-041-0-20	MAR-529-2.59				CME 55	84	974.0	972.5	1.5 C
22	B-042-0-20	MAR-529-2.59				CME 55	84	974.0	972.5	1.5 C
23	B-045-0-20	MAR-529-2.59				CME 55	84	976.0	974.5	1.5 C
24	B-046-0-20	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
25	B-049-0-20	MAR-529-2.59				CME 55	84	978.0	976.5	1.5 C
26	B-050-0-20	MAR-529-2.59				CME 55	84	979.0	977.5	1.5 C
27	B-053-0-20	MAR-529-2.59				CME 55	84	977.0	975.5	1.5 C
28	B-054-0-20	MAR-529-2.59				CME 55	84	976.0	974.5	1.5 C
29	B-057-0-20	MAR-529-2.59				CME 55	84	980.0	978.5	1.5 C
30	B-058-020	MAR-529-2.59				CME 55	84	980.0	978.8	1.2 C
31	B-061-0-20	MAR-529-2.59				CME 55	84	976.0	974.5	1.5 C
32	B-62-0-20	MAR-529-2.59				CME 55	84	975.0	973.5	1.5 C
33	B-065-0-20	MAR-529-2.59				CME 55	84	986.0	984.5	1.5 C
34	B-066-0-20	MAR-529-2.59				CME 55	84	987.0	985.9	1.1 C
35	B-069-0-20	MAR-529-2.59				CME 55	84	989.0	987.5	1.5 C
36	B-070-0-20	MAR-529-2.59				CME 55	84	987.0	985.8	1.2 C
37	B-073-0-20	MAR-529-2.59				CME 55	84	990.0	988.5	1.5 C
38	B-074-0-20	MAR-529-2.59				CME 55	84	990.0	989.0	1.0 C
39	B-077-0-20	MAR-529-2.59				CME 55	84	994.0	992.5	1.5 C
40	B-78-0-20	MAR-529-2.59				CME 55	84	995.0	994.0	1.0 C
41	B-081-0-20	MAR-529-2.59				CME 55	84	1000.0	998.5	1.5 C
42	B-082-0-20	MAR-529-2.59				CME 55	84	999.0	997.9	1.1 C
43	B-085-0-20	MAR-529-2.59				CME 55	84	1003.0	1001.5	1.5 C
44	B-086-0-20	MAR-529-2.59				CME 55	84	1005.0	1003.8	1.2 C
45	B-089-0-20	MAR-529-2.59				CME 55	84	997.0	995.5	1.5 C



V. 14.5 1/18/2019

	Boring	Sample	San	nple nth	Suba	grade	Stan	dard	НР		P	hysica	al Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate	Proble	m	Excavate an	d Replace	Recommendation
#			From	То	From	То	N ₆₀	N ₆₀₁	(tsf)	LL	PL	Ы	% Silt	% Clay	P200	Mc	MOPT	Class	GI	Content (ppm)	Unquitable	Unstable	Uncuitable	Unstable	(Enter depth in inches)
1	В	1	15	3.0	0.0	15	13		2	40	25	15	46	35	81	24	20	A-6a	10	100	Ulisuitable	N _{co} & M _C	Ulisuitable	12"	
-	001-0	2	3.0	4.5	1.5	3.0	10		1.5	53	21	32	35	51	86	25	18	A-7-6	19	100		HP & Mc			
	20	3	4.5	6.0	3.0	4.5	10	1	3.5							16	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	13	10	4.25							16	16	A-6b	16						
2	В	1	1.5	3.0	0.0	1.5	13			33	18	15	39	37	76	20	14	A-6a	10	220		N ₆₀ & Mc		12''	
	002-0	2	3.0	4.5	1.5	3.0	17		1.5	34	18	16	35	28	63	21	16	A-6b	8			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	17	1	3.5							14	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	29	13	2.5							16	16	A-6b	16						
3	В	1	1.5	3.0	0.0	1.5	11		1.5	34	18	16	40	22	62	19	16	A-6b	8	250		HP & Mc		12"	
	005-0	2	3.0	4.5	1.5	3.0	13		2	35	17	18	35	35	70	17	16	A-6b	10						
	20	3	4.5	6.0	3.0	4.5	18		2.5							14	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	8	8	1.5							15	16	A-6b	16						
4	В	1	1.5	3.0	0.0	1.5	13		3	34	17	17	26	53	79	18	16	A-6b	11	60					
	006-0	2	3.0	4.5	1.5	3.0	22		3.5	30	17	13	37	32	69	16	14	A-6a	8						
	20	3	4.5	6.0	3.0	4.5	29		4							15	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	28	13	4							16	14	A-6a	10						
5	В	1	1.5	3.0	0.0	1.5	14		2.5	40	21	19	51	34	85	18	16	A-6b	12	47					
	009-0	2	3.0	4.5	1.5	3.0	10		1.5	48	20	28	43	46	89	23	18	A-7-6	17			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	8		1.5							25	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	11	8	1.5							25	18	A-7-6	16						
6	В	1	1.5	3.0	0.0	1.5	13		3	37	19	18	38	39	77	24	16	A-6b	11	40		N ₆₀ & Mc		12"	
	010-0	2	3.0	4.5	1.5	3.0	13		2	43	19	24	35	42	77	23	18	A-7-6	14			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	20		4							14	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	25	13	4							14	14	A-6a	10						
7	В	1	1.5	3.0	0.0	1.5	8		1.5	40	21	19	33	32	65	17	16	A-6b	10	150		HP		12"	
	013-0	2	3.0	4.5	1.5	3.0	7		1.5	43	20	23	37	30	67	30	18	A-7-6	12			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	4		0.5							29	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	7	4	0.5							25	18	A-7-6	16						
8	В	1	1.5	3.0	0.0	1.5	15		0.75	36	19	17	22	21	43	21	16	A-6b	4	120		HP & Mc		18"	
	014-0	2	3.0	4.5	1.5	3.0	11									13	10	A-2-4	0			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	0			34	17	17	31	23	54	14	16	A-6b	7						
		4	6.0	7.5	4.5	6.0	44	0	0.5							16	16	A-6b	16						
9	В	1	1.5	3.0	0.0	1.5	11		1	29	21	8	43	29	72	23	16	A-4a	7	130		HP & Mc		12"	
	017-0	2	3.0	4.5	1.5	3.0	14		2	41	18	23	35	38	73	21	18	A-7-6	13			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	15		2.5							14	10	A-4a	8						
		4	6.0	7.5	4.5	6.0	31	11	3							14	10	A-4a	8						



V. 14.5 1/18/2019

#	Boring	Sample	Sam Dej	nple pth	Subg Dej	rade oth	Stan Penet	dard ration	НР		Р	hysica	l Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate	Proble	m	Excavate an (Item	d Replace 204)	Recommendation
#			From	То	From	То	N ₆₀	N _{60L}	(tsf)	ш	PL	PI	% Silt	% Clay	P200	Mc	M _{opt}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
10	В	1	1.5	3.0	0.0	1.5	11		1.5	29	20	9	50	26	76	19	15	A-4b	8	87	A-4b	HP & Mc	18''	12"	
	018-0	2	3.0	4.5	1.5	3.0	10		2	40	18	22	41	41	82	22	16	A-6b	13			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	10		1.5							20	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	18	10	3							16	16	A-6b	16						
11	В	1	1.5	3.0	0.0	1.5	11		1.5	32	20	12	43	24	67	18	15	A-6a	7	230		HP & Mc		12"	
	021-0	2	3.0	4.5	1.5	3.0	10		0.5	48	15	33	41	36	77	21	18	A-7-6	18			HP & Mc			
	02	3	4.5	6.0	3.0	4.5	6		0.5							22	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	13	6	1.5							25	18	A-7-6	16						
12	В	1	1.5	3.0	0.0	1.5	11		2.75	32	20	12	40	26	66	24	15	A-6a	7	240		N ₆₀ & Mc		12"	
	022-0	2	3.0	4.5	1.5	3.0	13			40	16	24	38	34	72	28	16	A-6b	13			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	15		3							17	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	11	11								22	16	A-6b	16						
13	В	1	1.5	3.0	0.0	1.5	14		2	38	20	18	38	36	74	10	16	A-6b	11	150					
	025-0	2	3.0	4.5	1.5	3.0	18		2	38	19	19	33	44	77	18	16	A-6b	12						
	20	3	4.5	6.0	3.0	4.5	25		3							15	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	26	14	4							15	16	A-6b	16						
14	В	1	1.5	3.0	0.0	1.5	17		2.5	38	20	18	37	32	69	17	16	A-6b	10	210					
	026-0	2	3.0	4.5	1.5	3.0	11		2	39	18	21	37	44	81	23	16	A-6b	12			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	70						20	6	26	14	10	A-2-4	0						
		4	6.0	7.5	4.5	6.0	31	11								16	10	A-2-4	0						
15	В	1	1.5	3.0	0.0	1.5	13		3.5	38	21	17	43	31	74	19	16	A-6b	11	350		N ₆₀ & Mc		12"	
	029-0	2	3.0	4.5	1.5	3.0	21		4.5	32	18	14	38	36	74	16	14	A-6a	9						
	20	3	4.5	6.0	3.0	4.5	24		4.5							17	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	28	13	4							18	14	A-6a	10						
16	В	1	1.5	3.0	0.0	1.5	14			37	21	16	34	28	62	23	16	A-6b	8	110		N ₆₀ & Mc		12"	
	030-0	2	3.0	4.5	1.5	3.0	17		3	39	19	20	36	39	75	19	16	A-6b	12			Mc			
	20	3	4.5	6.0	3.0	4.5	26		4							14	10	A-4a	8						
		4	6.0	7.5	4.5	6.0	33	14	4							15	10	A-4a	8						
17	В	1	1.5	3.0	0.0	1.5	13		2.5	32	22	10	43	36	79	17	17	A-4a	8	360					
	033-0	2	3.0	4.5	1.5	3.0	15		2.5	46	21	25	35	48	83	24	18	A-7-6	15			Mc			
	20	3	4.5	6.0	3.0	4.5	15		2.5							15	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	22	13	4.5							15	18	A-7-6	16						
18	В	1	1.5	3.0	0.0	1.5	11			41	26	15	42	29	71	16	23	A-7-6	9	370		N ₆₀		12"	
	034-0	2	3.0	4.5	1.5	3.0	8		0.5	51	19	32	36	36	72	22	18	A-7-6	17			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	8		1							23	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	8	8	0.5							25	18	A-7-6	16						

OHIO DEPARTMENT OF TRANSPORTATION

Subgrade Analysis

V. 14.5

1/18/2019

#	Boring	Sample	Sam De	nple pth	Subg De	grade pth	Stan Penet	dard tration	HP Physical Characteristics						Мо	Moisture Ohio DOT		Sulfate Problem Content		Problem Excavate and (Item 2		d Replace 204)	Recommendation		
"			From	То	From	То	N ₆₀	N _{60L}	(tsf)	ш	PL	PI	% Silt	% Clay	P200	Mc	M _{opt}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
19	В	1	1.5	3.0	0.0	1.5	10		2	41	26	15	52	30	82	26	23	A-7-6	10	390		N ₆₀ & Mc		12"	
	037-0	2	3.0	4.5	1.5	3.0	11		2.5	51	21	30	35	50	85	28	18	A-7-6	18			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	8		0.5							29	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	7	7	0.5							21	18	A-7-6	16						
20	В	1	1.5	3.0	0.0	1.5	11									13	6	A-1-b	0	310					
	038-0	2	3.0	4.5	1.5	3.0	10		0.5							12	14	A-6a	10			HP			
	20	3	4.5	6.0	3.0	4.5	13		1	37	22	15	36	39	75	15	17	A-6a	10						
		4	6.0	7.5	4.5	6.0	10	10	1.5	51	22	29	33	51	84	27	19	A-7-6	18						
21	В	1	1.5	3.0	0.0	1.5	13		2	33	20	13	38	41	79	18	15	A-6a	9	150		N ₆₀ & Mc		12"	
	041-0	2	3.0	4.5	1.5	3.0	21		4	43	20	23	34	45	79	22	18	A-7-6	14			Mc			
	20	3	4.5	6.0	3.0	4.5	31		4							15	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	35	13	4.5							13	14	A-6a	10						
22	В	1	1.5	3.0	0.0	1.5	15		2.5	34	18	16	30	40	70	17	16	A-6b	9	180					
	042-0	2	3.0	4.5	1.5	3.0	14		3	32	17	15	34	31	65	17	14	A-6a	8			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	17		3							15	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	18	14	2.5							16	14	A-6a	10						
23	В	1	1.5	3.0	0.0	1.5	10									16	14	A-6a	10	390		N ₆₀		12"	
	045-0	2	3.0	4.5	1.5	3.0	15		2	33	20	13	37	36	73	11	15	A-6a	9						
	20	3	4.5	6.0	3.0	4.5	20		2	31	18	13	35	40	75	25	14	A-6a	9						
		4	6.0	7.5	4.5	6.0	26	10	3							17	14	A-6a	10						
24	В	1	1.5	3.0	0.0	1.5	11		2.5	34	19	15	38	39	77	22	14	A-6a	10	410		N ₆₀ & Mc		12"	
	046-0	2	3.0	4.5	1.5	3.0	13		3	37	18	19	29	31	60	15	16	A-6b	9						
	20	3	4.5	6.0	3.0	4.5	14		3							15	16	A-6b	16						
		4	6.0	7.5	4.5	6.0	21	11	4.5							16	16	A-6b	16						
25	В	1	1.5	3.0	0.0	1.5	11		3	34	21	13	41	46	87	19	16	A-6a	9	140		N ₆₀ & Mc		12"	
	049-0	2	3.0	4.5	1.5	3.0	8									19	14	A-6a	10			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	7		1	51	20	31	38	51	89	27	18	A-7-6	18						
		4	6.0	7.5	4.5	6.0	6	6	0.5							26	18	A-7-6	16						
26	В	1	1.5	3.0	0.0	1.5	11									16	6	A-1-b	0	100					
	050-0	2	3.0	4.5	1.5	3.0	15									8	10	A-2-6	4						
	20	3	4.5	6.0	3.0	4.5	15		4	33	18	15	35	34	69	20	14	A-6a	9						
		4	6.0	7.5	4.5	6.0	24	11	4	33	18	15	36	36	72	14	14	A-6a	9						
27	В	1	1.5	3.0	0.0	1.5	11		1	33	18	15	36	48	84	23	14	A-6a	10	60		HP & Mc		12"	
	053-0	2	3.0	4.5	1.5	3.0	13		1.5	41	18	23	36	46	82	19	18	A-7-6	13			HP			
	20	3	4.5	6.0	3.0	4.5	21		3							16	10	A-4a	8						
		4	6.0	7.5	4.5	6.0	33	11	3.5							16	10	A-4a	8						



V. 14.5

1/18/2019

#	Boring	Sample	Sam Dej	nple pth	Subg Dej	rade oth	Stan Penet	dard ration	НР		Pł	iysica	l Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate	Proble	m	Excavate an (Item	d Replace 204)	Recommendation
#			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	Mc	M _{opt}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
28	В	1	1.5	3.0	0.0	1.5	11									24	10	A-2-4	0	60		N ₆₀ & Mc		12"	
	054-0	2	3.0	4.5	1.5	3.0	11		1							11	14	A-6a	10			HP			
	20	3	4.5	6.0	3.0	4.5	13		2	32	19	13	35	42	77	15	14	A-6a	9						
		4	6.0	7.5	4.5	6.0	21	11	3.5	37	19	18	37	35	72	19	16	A-6b	11						
29	В	1	1.5	3.0	0.0	1.5	14			38	23	15	45	32	77	19	18	A-6a	10	140					
	057-0	2	3.0	4.5	1.5	3.0	13			52	19	33	37	47	84	23	18	A-7-6	18			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	10		2							28	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	10	10	3							17	14	A-6a	10						
30	В	1	1.5	3.0	0.3	1.8	11		3	38	23	15	46	34	80	17	18	A-6a	10	160		N ₆₀		12"	
	058-0	2	3.0	4.5	1.8	3.3	10		1	50	21	29	32	42	74	25	18	A-7-6	17			HP & Mc			
	0	3	4.5	6.0	3.3	4.8	10		1.5	47	19	28	41	44	85	26	18	A-7-6	17						
		4	6.0	7.5	4.8	6.3	10	10	1.5							20	18	A-7-6	16						
31	В	1	1.5	3.0	0.0	1.5	8		1.5	29	19	10	37	32	69	17	14	A-4a	7	350		HP & Mc		12"	
	061-0	2	3.0	4.5	1.5	3.0	25		3	27	17	10	40	30	70	13	12	A-4a	7						
	20	3	4.5	6.0	3.0	4.5	28		4.5							13	10	A-4a	8						
		4	6.0	7.5	4.5	6.0	33	8	4.5							13	10	A-4a	8						
32	В	1	1.5	3.0	0.0	1.5	14		3	34	19	15	28	36	64	13	14	A-6a	8	360					
	62-0-	2	3.0	4.5	1.5	3.0	7		2	41	22	19	48	37	85	28	19	A-7-6	12			N ₆₀ & Mc			
	0	3	4.5	6.0	3.0	4.5	13		2							29	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	7	7	2							24	18	A-7-6	16						
33	В	1	1.5	3.0	0.0	1.5	11		2	35	19	16	35	35	70	25	16	A-6b	9	320		N ₆₀ & Mc		12"	
	065-0	2	3.0	4.5	1.5	3.0	14		2.5							17	16	A-6b	16						
	20	3	4.5	6.0	3.0	4.5	25		4	35	18	17	35	36	71	14	16	A-6b	10						
		4	6.0	7.5	4.5	6.0	29	11	4.5							16	16	A-6b	16						
34	В	1	1.5	3.0	0.4	1.9	15		2	35	19	16	40	42	82	22	16	A-6b	10	300		Mc			
	066-0	2	3.0	4.5	1.9	3.4	13		1.5	58	21	37	37	54	91	25	18	A-7-6	20			HP & Mc			
	20	3	4.5	6.0	3.4	4.9	14		2.5							21	18	A-7-6	16						
		4	6.0	7.5	4.9	6.4	15	13	3							19	18	A-7-6	16						
35	В	1	1.5	3.0	0.0	1.5	6		0.5	34	19	15	54	27	81	22	14	A-6a	10	200		HP & Mc		24"	
	069-0	2	3.0	4.5	1.5	3.0	8		0.5	51	19	32	51	36	87	25	18	A-7-6	18			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	8		1							24	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	13	6	2.5							18	18	A-7-6	16						
36	В	1	1.5	3.0	0.3	1.8	13		2	35	22	13	49	30	79	21	17	A-6a	9	240		N ₆₀ & Mc		12"	
	070-0	2	3.0	4.5	1.8	3.3	10		1.5	42	19	23	54	35	89	25	18	A-7-6	14			HP & Mc			
	20	3	4.5	6.0	3.3	4.8	7		0.5							21	18	A-7-6	16						
		4	6.0	7.5	4.8	6.3	7	7	0.5							22	18	A-7-6	16						



V. 14.5 1/18/2019

#	Boring	Sample	Sample Subgrade Depth Depth		rade pth	Stan Penet	dard ration	НР		Pł	nysica	l Chara	cteristics		Мо	isture	Ohio	DOT	Sulfate	Proble	m	Excavate an (Item 2	d Replace 204)	Recommendation	
#			From	То	From	То	N ₆₀	N _{60L}	(tsf)	LL	PL	PI	% Silt	% Clay	P200	Mc	M _{opt}	Class	GI	(ppm)	Unsuitable	Unstable	Unsuitable	Unstable	inches)
37	В	1	1.5	3.0	0.0	1.5	11		2	35	23	12	32	40	72	18	18	A-6a	8	53		N ₆₀		12"	
	073-0	2	3.0	4.5	1.5	3.0	13		1.5	49	21	28	38	50	88	24	18	A-7-6	17			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	7		1							28	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	11	7	2							22	18	A-7-6	16						
38	В	1	1.5	3.0	0.5	2.0	13		2.5	35	23	12	39	44	83	21	18	A-6a	9	33		N ₆₀ & Mc		12"	
	074-0	2	3.0	4.5	2.0	3.5	13		1.5	40	18	22	40	38	78	20	16	A-6b	13			HP & Mc			
	20	3	4.5	6.0	3.5	5.0	15		3							22	16	A-6b	16						
		4	6.0	7.5	5.0	6.5	21	13	2.5							21	16	A-6b	16						
39	В	1	1.5	3.0	0.0	1.5	13			29	24	5	38	28	66	12	19	A-4a	6	60					
	077-0	2	3.0	4.5	1.5	3.0	11		1.5	42	19	23	38	38	76	22	18	A-7-6	14			HP & Mc			
	20	3	4.5	6.0	3.0	4.5	11		2							22	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	13	11	2.5							17	18	A-7-6	16						
40	В	1	1.5	3.0	0.5	2.0	17			29	23	6	29	49	78	6	18	A-4a	8	40					
	78-0-	2	3.0	4.5	2.0	3.5	13		2.5	42	19	23	38	43	81	26	18	A-7-6	14			N ₆₀ & Mc			
	0	3	4.5	6.0	3.5	5.0	10		3							20									
		4	6.0	7.5	5.0	6.5	15	10	2							16									
41	В	1	1.5	3.0	0.0	1.5	14		1	36	20	16	51	25	76	16	16	A-6b	10	120		HP		12"	
	081-0	2	3.0	4.5	1.5	3.0	10		2	32	17	15	37	29	66	27	14	A-6a	8			N ₆₀ & Mc			
	20	3	4.5	6.0	3.0	4.5	10		2							17	14	A-6a	10						
		4	6.0	7.5	4.5	6.0	25	10	4							20	14	A-6a	10						
42	В	1	1.5	3.0	0.4	1.9	8		1	29	19	10	46	24	70	25	14	A-4a	7	140		HP & Mc		12"	
	082-0	2	3.0	4.5	1.9	3.4	10		1	53	20	33	40	48	88	18	18	A-7-6	19			HP			
	20	3	4.5	6.0	3.4	4.9	52		1.5							16	18	A-7-6	16						
		4	6.0	7.5	4.9	6.4	13	8	2							17	18	A-7-6	16						
43	В	1	1.5	3.0	0.0	1.5	10		2	29	18	11	55	19	74	23	14	A-6a	8	170		N ₆₀ & Mc		12"	
	085-0	2	3.0	4.5	1.5	3.0	14		2.5							16	16	A-6b	16						
	20	3	4.5	6.0	3.0	4.5	17		2	37	19	18	35	38	73	18	16	A-6b	11						
		4	6.0	7.5	4.5	6.0	32	10	4							16	16	A-6b	16						
44	В	1	1.5	3.0	0.3	1.8	14		4.5	52	25	27	29	19	48	19	20	A-4b	8	180	A-4b		21"		
	086-0	2	3.0	4.5	1.8	3.3	8			50	31	19	39	17	56	22	26	A-6b	9			N ₆₀			
	20	3	4.5	6.0	3.3	4.8	13		3							19	16	A-6b	16						
		4	6.0	7.5	4.8	6.3	14	8	2.5							18	16	A-6b	16						
45	В	1	1.5	3.0	0.0	1.5	13		2.5	30	18	12	35	38	73	22	14	A-6a	8	200		N ₆₀ & Mc		12"	
	089-0	2	3.0	4.5	1.5	3.0	15		2	47	20	27	34	47	81	21	18	A-7-6	16			Mc			
	20	3	4.5	6.0	3.0	4.5	24		4.5							15	18	A-7-6	16						
		4	6.0	7.5	4.5	6.0	25	13	4.5							15	18	A-7-6	16						



PID: 112818

County-Route-Section: MARC-529-2.59 No. of Borings: 45

Geotechnical Consultant:Resource International Inc.Prepared By:Peyman Majidi, PEDate prepared:4/5/2021

Chemical Stabilization Options											
320	Rubblize & Roll	No									
206	Cement Stabilization	Option									
	Lime Stabilization	Option									
206	Depth	14"									

Excavate and Replace										
Stabilization Option	ons									
Global Geotextile										
Average(N60L):	12"									
Average(HP):	0''									
Global Geogrid										
Average(N60L):	0''									
Average(HP):	0''									

Design CBR	5
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% Sampl	% Samples within 6 feet of subgrade													
N ₆₀ ≤ 5	1%	HP ≤ 0.5	8%											
N ₆₀ < 12	40%	0.5 < HP ≤ 1	7%											
12 ≤ N ₆₀ < 15	24%	1 < HP ≤ 2	30%											
N ₆₀ ≥20	22%	HP > 2	43%											
M+	31%													
Rock	0%													
Unsuitable	1%													

Excavate and Replace at Surface										
Average	0"									
Maximum	0"									
Minimum	0"									

% Proposed Subgrade Surface											
Unstable & Unsuitable	77%										
Unstable	74%										
Unsuitable	2%										

	N ₆₀	N _{60L}	HP	LL	PL	PI	Silt	Clay	P 200	Mc	M _{opt}	GI
Average	15	10	2.34	38	20	19	38	36	74	19	16	12
Maximum	70	14	4.50	58	31	37	55	54	91	30	26	20
Minimum	0	0	0.50	27	15	5	20	6	26	6	6	0

					Class	ificati	ion C	ount	s by	Sam	ple								
ODOT Class	Rock	A-1-a	A-1-b	A-2-4	A-2-5	A-2-6	A-2-7	A-3	A-3a	A-4a	A-4b	A-5	A-6a	A-6b	A-7-5	A-7-6	A-8a	A-8b	Totals
Count	0	0	2	4	0	1	0	0	0	15	2	0	45	51	0	58	0	0	178
Percent	0%	0%	1%	2%	0%	1%	0%	0%	0%	8%	1%	0%	25%	29%	0%	33%	0%	0%	100%
% Rock Granular Cohesive	0%					12%								88	3%				100%
Surface Class Count	0	0	2	2	0	1	0	0	0	7	2	0	26	26	0	24	0	0	90
Surface Class Percent	0%	0%	2%	2%	0%	1%	0%	0%	0%	8%	2%	0%	29%	29%	0%	27%	0%	0%	100%

GB1 Figure B – Subgrade Stabilization

