PROJECT DESCRIPTION

THE MOE-7-7.55 PROJECT CONSISTS OF A LANDSLIDE REPAIR ALONG STATE ROUTE 7 (SR 7) NEAR MILE MARKER 7.55 IN MONROE COUNTY, OHIO

HISTORIC RECORDS

A SEARCH OF THE AVAILABLE RECORDS ON ODOT'S TRANSPORTATION INFORMATION MAPPING SYSTEM (TIMS) REVEALED TWO PREVIOUS GEOTECHNICAL EXPLORATIONS PERFORMED AS PART OF THE MOE-7-2.06 PROJECT (ODOT, 1938) WITHIN THE CURRENT PROJECT LIMITS. HISTORIC BORING B-001-1-38 WAS LOCATED AT STA. 400+00, AND HISTORIC BORING B-04-3-38 WAS LOCATED AT STA. 403+00. BOTH BORINGS ENCOUNTERED A 6-INCH SURFICIAL LAYER OF TOPSOIL UNDERLAIN BY CLAY (CLASSIFIED AS A-7) TO THEIR TERMINATION DEPTH OF 4 FEET BELOW THE PREVIOUSLY EXISTING GROUND SURFACE (EL 643.3 AND EL. 641.2, RESPECTIVELY). A NOTE PROVIDED ON THE SOIL PROFILE SHEETS FOR THE MOE-7-2.06 PROJECT INDICATE A COMPACTION TEST WAS PERFORMED ON THE SOILS ENCOUNTERED IN BORING B-001-1-38, WITH A COMPACTION OF 97.9% ACHIEVED AT A MAXIMUM DRY UNIT WEIGHT OF 99.6 POUNDS PER CUBIC FOOT (PCF) AND AN OPTIMUM MOISTURE CONTENT OF 20.6%

GEOLOGY

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MONROE COUNTY LIES WITHIN THE UNGLACIATED ALLEGHENY PLATEAU PHYSIOGRAPHIC REGION OF SOUTHEAST OHIO. THE PHYSIOGRAPHIC FEATURES WITHIN THIS REGION HAVE BEEN INFLUENCED MAINLY BY PROCESSES OF EROSION AND UPLIFT. DRAINAGE-WAYS HAVE CUT STEEP, V-SHAPED VALLEYS AND NARROW RIDGETOPS THROUGHOUT MOST OF MONROE COUNTY, WITH THE EASTERN PERIPHERY OF THE COUNTY DRAINED EITHER INDIRECTLY OR DIRECTLY BY THE OHIO RIVER AND ITS TRIBUTARIES. THE PROJECT SITE IS LOCATED ALONG THE BANK OF THE OHIO RIVER, APPROXIMATELY 100 FEET NORTHWEST OF THE WATER'S EDGE.

THE SURFICIAL MATERIALS WITHIN MONROE COUNTY LARGELY CONSIST OF COLLUVIUM, RESIDUUM AND WEATHERED MATERIAL DERIVED FROM THE LOCAL BEDROCK. ALLUVIAL MATERIAL IS ALSO FOUND IN LOCALIZED AREAS AND NARROW BANDS ALONG STREAMS AND THE OHIO RIVER. THE BEDROCK MAPPED BELOW THE PROJECT SITE INCLUDES THE PENNSYLVANIAN-AGE MONONGAHELA GROUP. THE BEDROCK TYPES COMPRISING THIS GROUP ARE PREDOMINANTLY SHALE, SILTSTONE, AND MUDSTONE, WITH OCCASIONAL SECONDARY SEAMS OF SANDSTONE, COAL, AND THIN TO MEDIUM BEDS OF NON-MARINE LIMESTONE. COAL SEAMS OF NOTE WITHIN THE MONONGAHELA GROUP INCLUDE THE PITTSBURGH NO. 8, POMEROY (REDSTONE) NO. 8A, MEIGS CREEK (SEWICKLEY) NO. 9, UNIONTOWN NO. 10, AND WAYNESBURG NO. 11 COALS. NO SIGNIFICANT COAL MINING HAS BEEN RECORDED AT THE PROJECT SITE IN THE AVAILABLE MINE MAPS PUBLISHED BY THE OHIO DEPARTMENT OF NATURAL RESOURCES (ODNR).

RECONNAISSANCE

TWO PHASES OF EXPLORATION WERE PERFORMED AT THE PROJECT SITE, WITH THE INITIAL PHASE PERFORMED IN 2018 AND THE SECOND PHASE PERFORMED IN 2020. DURING PHASE 1, A VISUAL RECONNAISSANCE OF THE PROJECT SITE AND SURROUNDING RURAL AREA WAS PERFORMED ON JULY 10, 2018, AND DURING THE COURSE OF THE EXPLORATION BETWEEN JULY 27 AND AUGUST 6, 2018. DURING PHASE 2, SITE RECONNAISSANCES WERE PERFORMED ON JUNE 9 AND JULY 23, 2020, AND AGAIN DURING THE COURSE OF THE EXPLORATION BETWEEN AUGUST 10 AND 12, 2020. THE RECONNAISSANCES CONSISTED OF WALKING THE EXISTING ROADWAY AND SLOPE BELOW SR 7, AND NOTING ANY SIGNS OF RECENT SLOPE MOVEMENT, SEEPAGE OR EVIDENCE OF PRIOR SLOPE REPAIRS. SR 7 APPEARS TO HAVE BEEN CONSTRUCTED THROUGH A COMBINATION OF CUT AND FILL WITHIN THE PROJECT AREA. THE TERRAIN SURROUNDING THE PROJECT SITE GENERALLY SLOPES DOWNWARD FROM THE NORTHWEST TO SOUTHEAST AND IS COMPRISED OF CLEARED RESIDENTIAL AND HEAVILY-WOODED AREAS. THE OHIO RIVER IS LOCATED APPROXIMATELY 90 TO 120 FEET SOUTHWEST OF, AND ROUGHLY 30 FEET BELOW, SR 7 AT THE PROJECT SITE. THE HILLSIDE HAS BEEN CLEARED ON THE NORTHEAST SIDE OF SR 7 WITH SEVERAL RESIDENTIAL SRUCTURES NOTED, BEFORE GIVING WAY TO STEEPLY SLOPING, WOODED TERRAIN APPROXIMATELY 150 FEET FROM THE EDGE OF PAVEMENT.

SLOPE MOVEMENT WAS OBSERVED BELOW THE ROADWAY, WITH THE HEAD SCARP EXTENDING THROUGH A ZONE OF EXISTING GUARDRAIL ALONG THE NORTHBOUND LANE OF SR 7. APPROXIMATELY 70 FEET OF GUARDRAIL SHOWED SIGNS OF HORIZONTAL AND VERTICAL DISPLACEMENT, WITH THE SLIDE EXPOSING A 6-INCH DIAMETER GAS LINE IMMEDIATELY DOWNSLOPE OF THE DISTORTED GUARDRAIL. THIS AREA HAS BEEN TEMPORARILY SUPPORTED WITH VERTICALLY PLACED, HOLLOW 4.5-INCH DIAMETER PIPES SPACED ABOUT 3 TO 3.5 FEET ON CENTER, WITH ADDITIONAL SECTIONS OF GUARDRAIL SPANNING BETWEEN THE PIPES IN A PILE-AND-LAGGING TYPE SYSTEM. AS OF THE 2020 EXPLORATION, SEVERAL OF THESE PIPES WERE ANGLED AT APPROXIMATELY 30 TO 45 DEGREES FROM VERTICAL, AND A JERSEY BARRIER HAD BEEN PLACED ALONG A PORTION OF THE SCARP ON THE SHOULDER OF THE ROAD. THE SCARP EXTENDED JUST ABOVE THE DISTORTED GUARDRAIL POSTS AND HAD A VERTICAL DISPLACEMENT OF APPROXIMATELY 2 TO 3 FEET, WITH SECONDARY CRACKING OBSERVED IN THE NORTHBOUND TRAVEL LANE. THE FULL WIDTH OF THE LANDSLIDE SCARP WAS ESTIMATED TO BE ABOUT 150 FEET AS IT ARCED BEYOND THE GUARDRAIL TOWARD THE OHIO RIVER. IN ADDITION, AN APPROXIMATELY 20-FOOT STRETCH THE PAVED SHOULDER UPSLOPE OF THE LANDSLIDE SCARP APPEARED TO HAVE BEEN RECENTLY REPAIRED.

SUBSURFACE EXPLORATION

PHASE 1 OF THE GEOTECHNICAL EXPLORATION PROGRAM WAS CONDUCTED BETWEEN JULY 27, 2018 AND AUGUST 6, 2018 AND CONSISTED OF FOUR TEST BORINGS PERFORMED WITHIN THE NORTHBOUND TRAVEL LANE OF SR 7 (DESIGNATED AS BORINGS B-001-0-18, B-002-0-18, B-003-0-18, AND B-004-0-18). THE TEST BORINGS WERE DRILLED BY DHDC ENGINEERING CONSULTANTS, INC. UNDER THE GENERAL SUPERVISION OF AN HDR GEOTECHNICAL ENGINEER USING A MOBILE B-57 TRACK-MOUNTED DRILL RIG. THIS DRILL RIG WAS CALIBRATED ON FEBRUARY 27, 2018 AND HAD A DRILL ROD ENERGY RATIO OF 82.7%. PHASE 2 OF THE GEOTECHNICAL EXPLORATION PROGRAM WAS CONDUCTED BETWEEN AUGUST 10 AND 12, 2020 BY CENTRAL STAR DRILLING AND CONSISTED OF FOUR ADDITIONAL TEST BORINGS (DESIGNATED AS BORINGS B-002-1-20, B-002-2-20, B-004-1-20, AND B-004-2-20) PERFORMED ALONG THE SUPERVISION OF AN HDR GEOTECHNICAL ENGINEER UTILIZING A DIEDRICH D-50 TRACK-MOUNTED DRILL RIG CALIBRATED ON NOVEMBER 26, 2019 WITH A DRILL ROD ENERGY RATIO OF 86.8%. THE BORINGS WERE ADVANCED USING 3.25-INCH INTERNAL DIAMETER HOLLOW STEM AUGURS IN GENERAL ACCORDANCE WITH ODOT'S "SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS". SAMPLING OF THE SOILS WAS PERFORMED CONTINUOUSLY IN BORINGS B-002-0-18, B-002-1-20, B-003-0-18, AND B-004-1-20, AND AT 2.5-FOOT INTERVALS IN THE REMAINING BORINGS. SAMPLING WAS ACCOMPLISHED IN ACCORDANCE WITH THE "STANDARD TEST METHOD FOR PENETRATION TEST AND SPLIT-BARREL SAMPLING OF SOILS" (ASTM D 1586). UNDISTURBED SOIL SAMPLES WERE ALSO COLLECTED IN EACH OF THE BORINGS EXCEPT BORING B-004-2-20 IN ACCORDANCE WITH THE "STANDARD PRACTICE FOR THIN-WALLED TUBE SAMPLING OF SOILS FOR GEOTECHNICAL PURPOSES" (ASTM D 1587).

SAMPLING OF THE UNDERLYING BEDROCK WAS PERFORMED AT EACH BORING IN ACCORDANCE WITH THE "STANDARD PRACTICE FOR ROCK CORE DRILLING AND SAMPLING OF ROCK FOR SITE INVESTIGATIONS" (ASTM D 2113) USING AN NQ-SIZE DOUBLE-TUBE SWIVEL BARREL WITH DIAMOND BIT. INCLINOMETERS WERE INSTALLED IN BORINGS B-002-1-20 AND B-004-1-20 IN ACCORDANCE WITH ODOT'S "SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS".

	DESCRIPTION	ODOT CLASS	CLAS: MECH./	SIFIED 'VISUAL		H
	GRAVEL/STONE FRAGMENTS	A-1-a	1	2		e ^{xu}
	GRAVEL/STONE FRAGMENTS W/SAND	A-1-b	4	2		
	GRAVEL/STONE FRAGMENTS WITH SAND & SILT	A-2-4	12	8		
	COARSE & FINE SAND	A-3a	1	0		×
	SANDY SILT	A-4a	15	30		
* * * * * * * * * * * * * * *	SILT	A-4b	2	1		K
	SILT AND CLAY	A-6a	24	83		
	SILTY CLAY	A-6b	3	3		ļ
	ELASTIC CLAY	A-7-5	2	0		1 23
	CLAY	A-7-6	6	14		
		TOTAL	70	143		~
	SHALE	VISUAL				K
	CLAYSTONE	VISUAL				Ľ
	SILTSTONE	VISUAL				
	LIMESTONE	VISUAL				
	SANDSTONE	VISUAL				
XXXXX	PAVEMENT OR BASE = X = APPROXIMATE THICKNESS	VISUAL				40."
	SOD AND TOPSOIL = X = APPROXIMATE THICKNESS	VISUAL			BOULDEF	
$\mathbf{\Phi}$	BORING LOCATION - PLAN VIEW.					
(}	HISTORIC BORING LOCATION - PLAN VIEW.					
\bullet	INSTRUMENTED BORING LOCATION - PLAN VIEW.					
	DRIVE SAMPLE AND/OR ROCK CORE BORING PLOTTED T HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPH	O VERTICA	L SCALE	ONLY.		
+	AUGER BORING PLOTTED TO VERTICAL SCALE ONLY. HORIZONTAL BAR INDICATES A CHANGE IN STRATIGRAPH	HY.				
WC	INDICATES WATER CONTENT IN PERCENT.					
W	INDICATES FREE WATER ELEVATION.					
N60	INDICATES STANDARD PENETRATION RESISTANCE NORMALIZED TO 60% DRILL ROD ENERGY RATIO.					
(/Y/Z	NUMBER OF BLOWS FOR STANDARD PENETRATION TEST X= NUMBER OF BLOWS FOR FIRST 6 INCHES. Y= NUMBER OF BLOWS FOR SECOND 6 INCHES. Z= NUMBER OF BLOWS FOR THIRD 6 INCHES.	(SPT):				
:/Y/D*	NUMBER OF BLOWS FOR STANDARD PENETRATION TEST X= NUMBER OF BLOWS FOR FIRST 6 INCHES. Y/D"= NUMBER OF BLOWS (UNCORRECTED) FOR D" OF P	(SPT): ENETRATIO	N AT REF	USAL.	NO2	
SS	INDICATES A SPLIT SPOON SAMPLE.				4	INDI
ST	INDICATES SHELBY TUBE SAMPLE.				Ŭ	GREA
TR	INDICATES TOP OF ROCK.					
NP	INDICATES A NON-PLASTIC SAMPLE.				F	HIST
	INDICATES A PLASTIC MATERIAL WITH A MOISTURE CON	TENT				CLA

LEGEND

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EQUAL TO OR GREATER THAN THE LIQUID LIMIT MINUS 3.

INDICATES ROCK COMPRESSION TEST, ASTM D7012, METHOD C, RESULTS INDICATES SOIL UNCONFINED COMPRESSION TEST, ASTM D2166, RESULTS



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EXPLORATION FINDINGS

THE SOILS ENCOUNTERED ALONG SR 7 WITHIN THE PROJECT AREA GENERALLY CONSIST OF A LAYER OF EMBANKMENT FILL, UNDERLAIN BY COLLUVIUM, FINE-GRAINED ALLUVIAL SILTS AND CLAYS, AND RESIDUAL SOIL OVER SILTSTONE, SHALE, AND CLAYSTONE BEDROCK.

A SURFACE LAYER OF COMPOSITE PAVEMENT, RANGING IN THICKNESS FROM 8.5 TO 10 INCHES OF ASPHALTIC CONCRETE (ASPHALT) OVER 5 TO 6 INCHES OF PORTLAND CEMENT CONCRETE, WAS ENCOUNTERED IN ALL OF THE PHASE 1 ROADWAY BORINGS EXCEPT BORING B-001-0-18, WHERE 7.5 INCHES OF ASPHALT WAS ENCOUNTERED. EMBANKMENT FILL WAS ENCOUNTERED DIRECTLY BENEATH THE PAVEMENT SECTION IN ALL OF THE PHASE I ROADWAY BORINGS EXCEPT BORING B-001-0-18, WITH THICKNESSES RANGING FROM ABOUT 1.8 TO 3.9 FEET. THE FILL WAS COHESIVE IN NATURE, AND COMPRISED PRIMARILY OF STIFF SILT AND CLAY (A-6A) AND CLAY (A-7-6).

COLLUVIUM WAS ENCOUNTERED BENEATH THE PAVEMENT IN BORING B-001-0-18, BENEATH THE FILL IN THE REMAINING PHASE 1 BORINGS, AND FROM THE GROUND SURFACE IN BORINGS B-002-1-20, B-002-2-20, AND B-004-1-20 WITH THICKNESSES RANGING FROM APPROXIMATELY 6 TO 34 FEET. THE COLLUVIUM CONSISTED OF PREDOMINANTLY STIFF TO VERY STIFF SILT AND CLAY (A-6A), SILTY CLAY (A-6B), ELASTIC CLAY (A-7-5), AND AND (A-7-5), AND AND AND (A-7-5), AND AND (A-7-5), AND (A-(A-7-6) WITH MEDIUM STIFF INTERVALS ENCOUNTERED IN BORINGS B-002-1-20 AND B-003-0-18.

ALLUVIUM WAS ENCOUNTERED IN EACH OF THE BORINGS BENEATH THE COLLUVIUM SOILS, OR FROM THE SURFACE IN BORING B-004-2-20, WITH THICKNESSES RANGING FROM ABOUT 13.5 TO 25.5 FEET. THE ALLUVIUM CONSISTED OF GENERALLY SOFT TO STIFF SANDY SILT (A-4A), SILT AND CLAY (A-6A), AND SILTY CLAY (A-6B), WITH LOOSE TO MEDIUM DENSE GRAVEL WITH SAND (A-1-B), GRAVEL WITH SAND AND SILT (A-2-4) AND SILT (A-4B) ENCOUNTERED TO A LESSER EXTENT. RESIDUUM ENCOUNTERED BENEATH THE ALLUVIUM RANGED FROM ABOUT TO 17.5 FEET IN THICKNESS AND CONSISTED OF COHESIVE SOILS CLASSIFIED AS HARD SANDY SILT (A-4A) AND SILT AND CLAY (A-6A) OR MEDIUM DENSE TO VERY DENSE GRAVEL (A-1-A), SRAVEL WITH SAND (A-1-B), GRAVEL WITH SAND AND SILT (A-2-4), AND COARSE AND FINE SAND (A-3A). THE N-VALUES GENERALLY INCREASED WITH DEPTH, ENCOUNTERING SPLIT SPOON REFUSAL (> 50 BLOWS WITHIN A 6-INCH INTERVAL) NEAR THE BOTTOM OF THE RESIDUAL LAYER.

BEDROCK PREDOMINANTLY CONSISTED OF LAYERS OF SHALE AND SILTSTONE. ISOLATED LAYERS OF CLAYSTONE WERE ENCOUNTERED IN BORINGS B-002-0-18, B-002-1-20 AND B-003-0-18 AT DEPTHS RANGING FROM APPROXIMATELY 46 TO 67.5 FEET BELOW EXISTING GRADE. THE CLAYSTONE WAS TYPICALLY ENCOUNTERED IMMEDIATELY BENEATH THE OVERBURDEN SOILS AND RANGED FROM ABOUT 5 FEET TO 8 FEET IN THICKNESS, WITH A RECORDED ROD VALUE OF 40%. AN ISOLATED 0.8-FOOT THICK SEAM OF LIMESTONE WAS ENCOUNTERED IN BORING B-002-20 AT A DEPTH OF ABOUT 34.4 FEET, WITH AN ROD OF 55%. THE SHALE RANGED FROM ABOUT 3 TO 30 FEET IN THICKNESS WITH RECORDED RQD VALUES BETWEEN 14% AND 54%. THE SILTSTONE RANGED FROM ABOUT 4.0 TO 11 FEET IN THICKNESS, WITH RECORDED ROD VALUES RANGING FROM 0% TO 89%. ISOLATED SEAMS OF SANDSTONE WERE OBSERVED WITHIN THE SILTSTONE, AND AN 0.7-FOOT THICK LAYER OF SANDSTONE WAS ENCOUNTERED AT THE BOTTOM OF BORING B-004-2-20, WHICH HAD A RECORDED ROD OF 100%.

WITH THE EXCEPTION OF BORING B-001-0-18, GROUNDWATER WAS ENCOUNTERED DURING DRILLING AT DEPTHS RANGING FROM 8.5 TO 52.5 FEET. DELAYED WATER READINGS WERE NOT OBTAINED AS THE BORINGS WERE GROUTED AND SEALED IMMEDIATELY UPON COMPLETION, OR COMPLETED AS AN INCLINOMETER. DUE TO THE LOW PERMEABILITY OF COHESIVE SOILS AND NEAR-IMMEDIATE BACKFILL OF THE BORINGS UPON COMPLETION, THE LACK OF GROUNDWATER ENCOUNTERED IN THE BORINGS MAY NOT BE REPRESENTATIVE OF LONG-TERM CONDITIONS. GROUNDWATER LEVELS MAY VARY THROUGHOUT THE YEAR, DEPENDING ON PRECIPITATION, DIVED LEVEL AND OTHER SEASONAL VARIATIONS. RIVER LEVEL, AND OTHER SEASONAL VARIATIONS.

<u>SPECIFICATIONS</u> THE GEOTECHNICAL EXPLORATION WAS PERFORMED IN GENERAL ACCORDANCE WITH THE STATE OF OHIO, DEPARTMENT OF TRANSPORTATION, OFFICE OF GEOTECHNICAL ENGINEERING "SPECIFICATIONS FOR GEOTECHNICAL EXPLORATIONS", DATED JULY 2017.THE PHASE 2 EXPLORATIONS WERE PERFORMED IN ACCORDANCE WITH THE UPDATED SPECIFICATIONS DATED JULY 2020.

AVAILABLE INFORMATION ALL AVAILABLE SOIL AND BEDROCK INFORMATION THAT CAN BE CONVENIENTLY SHOWN FROM THE GEOTECHNICAL EXPLORATION SHEETS HAS BEEN SO REPORTED. ADDITIONAL EXPLORATIONS MAY HAVE BEEN PERFORMED TO STUDY SPECIFIC ASPECTS OF THE PROJECT. COPIES OF THIS DATA, IF ANY, MAY BE INSPECTED AT THE ODOT DISTRICT 10 OFFICE LOCATED AT 338 MUSKINGUM DRIVE, MARIETTA, OHIO 45750.

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DRAWN CLW	CHECKED DMV
SOIL PROFILE - LANDSLIDE	COVER SHEET
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PROJECT: MOE-7-07.55 DRILLING FIRM TYPE: LANDSLIDE SAMPLING FIRM PID: 108676 SFN: DI: 108676 SFN:	/ OPERATOR: DHDC / A.U. DRILL RIGMOBILE B-57 TRACK RIG STATION / OFF // LOGGER: HDR / S. REED HAMMER: AUTOMATIC HAMMER // LOGGER: HAM CALIBRATION DATE: 227/18	=SET: <u>398+71, 13' RT.</u> EXPLORATION I SR-7 642.3 (MSL) EOB: <u>84.0 ft.</u> PAGE	Θως
	HOU: SPT/ST/NG ENERGY RATIO (%): 82./ LAT/ LONG: ELEV. DEPTHS SPT/ N ₈₀ REC SAMPLE HP GRADATION (%) 1.100 DEPTHS SPT/ N ₈₀ REC SAMPLE APP GRADATION (%)	39.010341, -80.931318 1.01 2 ATTERBERG 0D07 HOLE CLASS (G) CEALE	<u>ч щ й</u>
ASPHALT PAVEMENT (7.5 INCHES) ASPHALT PAVEMENT (7.5 INCHES) VERY STIFF, REDDISH-BROWN, TRACE TAN, SILT AND CLAY, SOME SAND, TRACE GRAVEL, DAMP			3
	- 2 - 4 4 17 50 SS-1 3.50	19 A-6a (V)	_
@ 4.0' - 5.5' : grab sample obtained from auger spoils		18 A-6a (V)	_
@ 6.5' : orange-brown and tan	- 6 7 - 3 4 7 15 67 SS-3 4.00	16 A-6a (V)	_
	- 9	37 34 22 12 18 A-6a (7)	
@ 11.5' : brown, trace tan, and gray Below 11.5' : some gravel and stone fragments	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	19 A-6a (V)	
		17 A-6a (V)	
	- 17 - 17 - 18	17 A-6a (V)	
	- 19 9 6 22 83 SS-8 2.50	16 A-6a (V)	_
	$\begin{bmatrix} -21 \\ -22 \\ -23 \\ -23 \\ -23 \\ -23 \\ -10 \\ 22 \\ 67 \\ 8S-9 \\ 2.00 \\ -2 \\ -2 \\ -2 \\ -2 \\ -2 \\ -2 \\ -2 \\ $	19 A-6a (V)	_
	- 24	16 A-6a (V)	_
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	24 A-6a (V)	
ESCRIPTIONS)	- 29 - 29 - 30 - 30 - 30 - 6 - 19 - 94 	19 A-6a (V)	_
MEDIUM STIFF TO STIFF, OLIVE-BROWN TO GRAY, SANDY SILT, SOME CLAY, WET	611.3 -31 - 31 - 10 100 SS-13 0.75 -	25 A-4a (V)	_
 33.' - 35.0' : Shelby tube was attempted from 33.0' to 35.0', but no sample was recovered. SPT sample was then driven between 33.0' and 34.5'. 	- 33 3 3 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	23 A-4a (V)	_
@ 35.5': Qu = 2643 psf		31 24 18 6 22 A-4a (8)	_
25_28.7-5-30		23 A4a (V)	_
MO MEDIUM STIFF TO STIFF. OLIVE-BROWN TO GRAY.			
SILT AND CLAY, LITTLE SAND, MOIST	-41 2 3 7 100 SS-17 0.50	24 A-6a (V)	
@ 42.5': Qu = 2320 psf	- 43 - 43 - 43 - 44 - 92 ST-18 0.75 0 1 16 47	36 31 19 12 26 A-6a (9)	_
KE MEDIUM STIFF, BROWN AND GRAY, SANDY SILT, SOME CLAY, WET	-45 -0 2 8 100 SS-19 1.00	25 A-4a (V)	_
0- 52:60 03:25 - C	-47 0 3-20 0.75 0 1 33 39	27 24 17 7 24 A.4a (6)	_
21 - 709.700	-51 -51 - 51 - 51 - 51 - 51 - 51 - 51 -	24 A.4a (V)	_
но - (21 х 11	-52 0 4 100 SS-22 0.75 - - -	29 A-4a (V)	_
вокие гос ($\begin{array}{c ccccccccccccccccccccccccccccccccccc$	32 A4a (V)	_
HARD, REDDISH-BROWN, SANDY SILT, SOME CLAY, DAMP	585.8 - 30 - 57 - 33 - 57 - 33 - 58 58 - 58 58	21 24 18 6 15 A.4a (5)	
SHALE, REDDISH-BROWN AND GRAY, HIGHLY WEATHERED. VERY WEAK. FISSILE.		13 Rock (V)	
2 MOE-7-7,55	SOIL PROFILE - LANDSLID	ш	DRAWN CLW CLW
34	BORING LOG B-001-0-	18	DMV

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1-0-18	HOLE							
B-00	DDOT ASS (GI)	1	ock (V)	ORE	ÖRE	ORE	ORE	ORE
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PG 2	≥ ₽ ⊒		1					
3/18 			•					
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ĒND	(%) V (%) V		1					
8/3/18	DATIO		1					
ART:	GRA GRA		· ·					
ST/	(tsf)		•					
13' RT.	MPLE ID		5S-26	4Q-1	NQ-2	e Q	NQ-4	5 Q
98+71,	KEC SA		0	06	96	00	96	100
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OFFSE	SPT/ RQD	q	42 50/1"	0	25	25	35	36
TION /	s	- 61 - 62	- 63	- 64 - 65	- 66	- 68 - 70 - 71 - 72 - 73	- 74	- / 3 - / 3
STA	ОЕРТН							
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7-07.55	ELEV 582.3		578.3	200			563.3	558.3
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	ESCRIF OTES GRAY,	SILE. (c		D 26%, HERE	ED TO TELY V VAINED RFACE	(r am)		RED, SI UGH S UGH S C 100%
	AND N AND N	NK, FIS		%) AND ED, RQ WEAT	MINATE DDERA INE GF GH SUI	lle sear tone se		EATHEI LAMIN ED, RO 6%, RE 6%, RE
	MATE: 3ROWN	RY WEA		ALE (54 \CTURI +IGHLY	ED, LA AY, MC EAK, F Y ROU	psi (sha psi (silt		
SFN	DISH-E	D, VEF		ED SH/ LY FR/ SRAY, F	grain Ne , gr (to w Jghtl Jints.	= 156 F		GRAY NE GR LY FR/ DINTS;
108676	.E, REC	THERE		ERATE HALE, G	Y FINE LTSTO Y WEAL DED, SI DING J(.3' : Qu 5' : Qu		STONE DNG, FI ERATE DING J(
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	CLW CHECKED DMV	
REHOLE AT 35.5 FEET TO FACILITATE PISTON TUBE SAMPLING NITTLES: BACKFILLED WITH 4 BAGS BENTONITE GROUT: MIXED 80 GAL. WATER	SOIL PROFILE - LANDSLIDE BORING LOG B-001-0-18	
DIES: INTRODUCED WATER INTO THE BOF	MOE-7-7.55	
2/8r/sr - тар.тоа но - (7r X rr) бол вояме Log (7r X rr) БД	8/34	

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PROJECT: MOE-7-07.55 DRILLING FIRM / OI TYPE: LANDSLIDE SAMPLING FIRM / I DID: 108676 SEN'	PERATOR: DHDC/A.I LOGGER: HDR/S.REE D: 3.25" HSA/NO		AMME		BILE B-57 TOMATIC	TRACK		STAT ALIGI			SET:	400 SI	+47, 7-7	10' RT	83 0 #	LORAT 3-002-0	10N II -18 AGE
START: 8/1/18 END: 8/2/18 SAMPLING METHO	D: SPT/ST/NQ		LERG	RATI	0 (%):	82.7			LON	U U		9.616	785, -	80.93	0775	<u> </u>	OF 2
MA I EKIAL DESCRIPTION AND NOTES	643.2 DEPTHS	1 2 2 2 2 2 2	z″ ≥∩	й % °		т (tsf)	9 19	N N N N	P IO	%) N	< て			≷ _ و	CLAS	ot s(GI) SI	HOLE
ASPHALT PAVEMENT (9 INCHES) CONCRETE PAVEMENT (6 INCHES) STIFE PAVININ SIIT AND CLAY SOME SAND LITTLE	642.5 642.0	-														****	
GRAVEL DAMP (FILL) @ 1.5' - 3.0': grab sample obtained from augers	640.2	2 5 4	5 (1)	0	SS-1	•	18	18	10	23	31	34	2	15	9-9-9	a (5)	
STIFF, REDDISH-BROWN, SILT AND CLAY, SOME TO "AND" SAND, TRACE GRAVEL, DAMP		0 4 4 8	ي ت	26	SS-2	2.50								52	A-6	() E	
		5 2 3	4 5	44	SS-3	1.50								25	9-6	() ()	
		6 7 9 7 3	- - -	72	SS-4	1.75								5	A-6) S	
		8 3 3	4 5	20	SS-5	1.50								5	A-6) S	
Below 9.0' : some stone fragments		9 2 10 3 3	4 5	0 72	SS-6	2.25								5	A-6	() a	
		11 2 4	5	56	SS-7	1.75								-15	A-6	() E	
		12 2 13 5 5	-7 2	83	8-SS	1.50								÷	A-6) S	
		14 2 3	2 -	1 78	8-SS-9	1.25								-10	A-6) ()	
@ 16.5' : sandstone cobbles		16 2 3	2 -	72	SS-10	0 1.50								51	A-6	() ()	
)		17	'] {		SS-1	1 1.50	•]	•						F	A-6	(V) E	
@ 18.0' : sandstone fragments embedded in clayey soil matrix		18 30 19 12	26	\$ 78	SS-12	-								4	A-6	() ()	
@ 19.5' : sandstone fragments embedded in clayey soil matrix		20 4 4	-7 9	t 61	SS-1:	3 2.50								4	A-6) S	
		21 3 22 5 5	9	67	SS-1	4 2.25								15	9-P) S	
		23 23 15	94 14	94	SS-1!	5 1.25	4	30	9	27	33	32	0	2 17	A-6	a (6)	
		24 2 25 6	9	78	SS-16	3 1.50								18	A-6) S	
		26 3 28	5; 10	56	SS-1	7 1.50	~	19	2	33	64	32	0	11	A-6	a (8)	
2)/01/1		2/ 5 28 6	5 0	20	SS-18	3 1.50	,				,			18	8-6	() ()	
		29 4 4	9	t 56	SS-19	9 1.50								- 20	A-6	() S	
STIFF TO VERY STIFF. MOTTLED BROWN AND GRAY	612.3	30 5 31 8	к б	3 72	SS-20 SS-20	A 1.75 B 3.00	· ·							- 25	A-6	55	
SILT AND CLAY, TRACE SAND, TRACE GRAVEL, MOIST		32 36	7 7	100) SS-2	1 1.75	0	-	9	50	43	35 2	-	23	A-6	(10)	
6 @ 33.0' - 35.0' : Qu = 4036 psf		34		96	ST-22	2 1.00	.	-	~	52	39	31	6	2 18	A-6	a (9)	
SOFT TO MEDIUM STIFF, GRAY, SILT AND CLAY, SOME SAND, TRACE GRAVEL, MOIST		35 3 36 4 36 4	2 7	5 100) SS-20	3 1.00								25	9-9	() ()	
107 <u> </u>		37 - 3 4	5	5 100) SS-24	4 0.75								22	A-6	() E	
[1] @ 38.0' - 40.0': Attempted undisturbed shelby tube, but did pot recover sample. An SPT sample was then driven from 38.0' to 39.5'		39		94	ST-2	5 1.00								23	4-6	(V) E	
C //001 07202		40 0 2 41	3 7	94	SS-26	9 1.00			•		,			53	A-6	() ()	
@ (@ 41.5' - 43.5': Attempted undisturbed shelby tube, but did not recover sample. An SPT sample was then driven from (141.5' to 43.0'.		42		10() ST-21	7 0.50	0	0	20	48	32	58	7	1 25	A-6	a (8)	
		44 44 0	ω 4	10() SS-28	3 1.00			1	1	,			23	8-6	() ()	
		45 <mark>2</mark> 5	5	t 100) SS-2(9 1.00		-	ı			-		24	H-6	(V) E	
					SS-3(0 75			,	ſ		<u> </u>	<u> </u>	3	A_6	2	

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	A-6a (V)	A-6a (V)	A-6a (V)	A-6a (V)	A-6a (8)	A-4a (V)	A-4a (2)	A-4b (8)	Rock (V)	Rock (V)
	23	24	26	28	29	32	22	14	7	8
		•	•		7		ЧN	ω	•	
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	0.75	0.75	0.75	1.25	0.50	•		4.50	•	'
	SS-30	SS-31	SS-32	SS-33	SS-34	SS-35	SS-36	SS-37	SS-38	SS-39
	100	100	100	100	100	100	100	33	100	100
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16	- 47		43 50	- 51 - 51	23	- 54 - 54		- 57	58 	3
					590.7				۲	
					3				<u> </u>	
					589.7		586.7	205.0	7.000	
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77								++++	+ 11/11	
						VED WILT, LITTLE VED WITH THIN BEDS. WE		E SAND, TRACE	DDISH-BROWN,	
			ıl sandy silt seams			M DENSE, GRAY, SA AVEL, SLIGHTLY VAR Y LAMINATIONS ANE		Γ, "AND" CLAY, LITTLI	WNISH-GRAY TO RE RED, VERY WEAK.	
			@ 49.0' : occasiona			LOOSE TO MEDIUI CLAY, TRACE GRA OCCASIONAL CLA'		HARD, GRAY, SILT GRAVEL, DAMP	CLAYSTONE, BROM HIGHLY WEATHER	
w	55 - C:/P	:60 0Z/8L	GDT - 12/	.TOO HO	- (21 X I	l) no no li	ור פסצוא	DOT SC	O ARAO	INATS

Ň DMV B-002-0-18 - LANDSLIDE SOIL PROFILE -BORING LOG **MOE-7-7**.55 9/34

02-0-18	HOLE			1								
DF 2 B-0(ODOT CLASS (GI)		Rock (V)	CORE	CORE	CORE	CORE	CORE				
PG 2 (Š	α										
N. PROJECT: MOE:70755 STATION / OFFSET: 400-47.10° RT. STATI. STATI.												
8/2/1			· ·									
END:	%) CL		, ,									
18 E) IION (
8/1/	CS F		· ·									
TART	U B		, ,									
	E HP (tsf)		, , , ,]								
47, 10' R ⁻	SAMPLI	07 00	5S-41	ğ	ğ	ğ	ğ	ğ				
400+7	REC (%)			100	87	100	80	100				
SET:	°°Z ∠0		· ·	1								
I / OFF	R B B	-	20/3	40	22	<u>.</u>	2 7	0				
STATIO	DEPTHS	 61					7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7					
1.55	ilev. 83.2		80.2		1.77.1		64.4	60.2				
MOE-7-0	Шю											
PROJECT:	PTION	EDDISH-BROWN, continued)		ERELY TO HIGHLY E GRAINED, DNAL CLAY SEAMS, IRFACES ALONG %.	IGHTLY RONG, VERY FINE BEDDED, COUS SHALE, GHTLY S ALONG BEDDING			JERATELY RONG, FINE THIN SHALE WITH SLIGHTLY JOINTS; RQD 0%,				
: 108676 SFN:	TUBBE/IS JUNE (INCOME) Introver ILY WEATHERED, VERY WEAK. (continued) AND NOTES SETONE, BROWNISH-BROWN, SEVERELY TO THERED, VERY WEAK. (continued) ITHERED, VERY WEAK. (continued) THERED, VERY WEAK. (continued) ITHERED, VERY WEAK. (continued) THERED, VERY WEAK. (continued) ITHERED, VERY WEAK. Continued) THERED, VERY WEAK. (continued) ITHERED, VERY WEAK. CONSTINCT CLUCTURED WITH SILICKENSIDED SURFACES SURGACIONS SIGNAL SILICATIVA ITHERED, WEAK TO SILIGHTLY STRONG, N ING GRAINED, THIN TO MEDIUM BEDDED SOLAREOUS SIGNAL SILICATIVA STRONAL SILTSTONE SEAMS. SILIGHTLY STRONG, N STONAL SILTSTONE SEAMS. SILIGHTLY STRONG, N STONEL GRAY, SLIGHTLY STRONG, STRONG, STRONG, STRONG, STRONG, STRONG, STRONG, STRONG, STRONG,											

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	DRAWN CLW CHECKED DMV
NTTES. MOCKILLED WITH 1 BASS BENTONTE CROIT. MXED 80.001. WXED 80.001.	SOIL PROFILE - LANDSLIDE BORING LOG B-002-0-18
ES: NONE NDONMENT METHODS, MATERIALS, QU	MOE-7-7.55
L9D.(2001Т91ЯЭ283 ФЭТАФЧ) 2001 ФЭТҮТ_АА-01 7102_82.7-7-ЭОМ_06708102/8206781/012АЭ/ЮНХИОЧ/Э - 35:60 05/81/51 - ТФЭ.ТОФ НО - (71 X 11) 2001 ВИКОР ФОД ВАСАТОРИАТА СОСТОЯНОВИТОРИСТИИ СТОР СТОВ 102/02/02/02/02/02/02/02/02/02/02/02/02/0	10/34

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PROJECT: MOE-7-07.55 DRILLING FIRM / C	PERATOR: CE	ENTRAL STAR / TS	S DRILL	S.	DIED	RICH D	-50	ST	ATIO	N 0F	ESE		00+3	9, 62' I	RT.	EXPLORA	TION ID
TYPE: LANDSLIDE SAMPLING FIRM / PID: 108676 SFN: DRILLING METHOD	LOGGER:):3.25"	HDR / AKB ' HSA / NQ2	CALIBF	ER: DI	EDRICH	11//	00110 26/19	E F	IGNM EVAT	ENT: ION:	627.	7 (MS	SR 7 SL) E	 Bi	64.	B-002- 5 ft.	1-20 PAGE
START: 8/12/20 END: 8/12/20 SAMPLING METHC	D: SPI	r / ST / NQ2	ENERG	Y RAT	(%) OL		8.0 10	Ϋ́.		UDN NO	17	39.6	1665	- 80	93070		1 OF 2
AND NOTES AND NOTES	627.7	DEPTHS		1 ⁶⁰ (9			tsf)	5 U 5 4			5	2 =		2 =	NC N	ODOI CLASS (GI)	INCL.
STIFF TO VERY STIFF, BROWN, CLAY , SOME SILT, LITTLE GRAVEL, TRACE SAND, DAMP			ы 6 4	8	ი ი	<u>.</u>	.75		1	I	1	,	I	ı.	24	4-7-6 (V)	
	624.7		6 4 3	0	0 0	5-2	75 1	20	2	34	47	51	25	26	20 4	-7-6 (17)	
MEDIUM STIFF TO STIFF, BROWN, ELASTIC CLAY, "AND" SILT, TRACE GRAVEL, TRACE SAND, DAMP	623.2	0 4	2 2 2	- 0	7	ب ب	. 25		т С	35	58	61	30	31	19	-7-5 (20)	
MEDIUM STIFF TO STIFF, RED-BROWN, CLAY , "AND" SILT, LITTLE GRAVEL, TRACE SAND, DAMP			2 3 3	7 6	- vi vi	4-0	.75	<u>'</u>	'	·	'	,	•		52	4-7-6 (V)	
			2 3 4	6	4	5-5	.25		'	'	'				23	4-7-6 (V)	
			2 3 4	10	8 2	9-9	1	4	e	36	43	44	23	21	21 4	-7-6 (13)	
	617.2	∑ 2 2 2	2 3 9	3	0 0	2-2	00		'	1	'	•			19	4-7-6 (V)	
STIFF TO VERY STIFF, BROWN TO RED-BROWN, CLAY, "AND" SILT, TRACE GRAVEL, TRACE SAND, DAMP		; ; ;	2 2 2	10	й О	به مې	00.	'	'	•	'	•			25 /	4-7-6 (V)	
		☐ <u>7</u> 7	2 2 2	16	й О	6-0	25	'	1	1	•				23	4-7-6 (V)	
		14		~	6 0	-10		-	-	52	46	50	26	24	22	r-7-6 (16)	
		, <u>9</u>	6 7 9	23 10	SS O	11	.25	<u>'</u>	'		•	,	,		23	A-7-6 (V)	
			567	10	SS OC	-12	00.		'	1	'	•			23	4-7-6 (V)	
	607.7	- <u>-</u>	4 4 6	11	0 S	-13	.75		'	•	'	•	•	•	23	A-7-6 (V)	
STIFF, BROWN, SILTY CLAY, TRACE SAND, MOIST	606.2	- 20 - 21 - 21	8 8 4 7	9	00	14	50		en en	58	39	40	24	16	27	A-6b (10)	
VERY SOFT TO SOFT, BROWN, SILT AND CLAY, SOME SAND, MOIST		- 23	е С 4	10	SS 00	-15 0	.50	· ·	'	1	'				25	A-6a (V)	
	603.2	- 23 + - 24 -	· · · · · · · · · · · · · · · · · · ·	0	SS O	-16	.50 (5	47	32	32	19	13	23	A-6a (9)	
MEDIUM STIFF TO STIFF, BROWNISH-GRAY, SANDY SILT, SOME CLAY, MOIST			2 3 4	0	83 00	-17	.50	<u>'</u>	'	·	·				24	A-4a (V)	
				9	LS 00	-18		0	56	47	28	29	19	6	24	A-4a (8)	
MEDIUM STIFF, BROWNISH-GRAY, SILT , SOME CLAY, LITTLE SAND, WET	7.86C	W 599./ 28	2 2 2	6 10	8	-19		<u> </u>	'	'	'				23	A-4b (V)	
	***** ***** ***** 596.7	3 3	2 2 2	6 10	88 00	-20	-		4	53	59	29	19	10	27	A-4b (8)	
SOFT TO MEDIUM STIFF, BROWNISH-GRAY, SANDY SILT, SOME CLAY, WET		33 2	3 0 0	4 10	SS OC	-21	<u>0</u>	'	'	ı	'		,	,	28	A-4a (V)	
	593.7	33	2 2 3	7 10	00	-22 0	.75 (0	24	46	30	29	19	10	27	A-4a (8)	
STIFF, GRAY, SANDY SILT, SOME CLAY, WET 2 @ 34.0' - 40.5': contains sand seams		35 7	2 2 4	9 10	00	-23		<u>'</u>	'	1	'	,	·		27	A-4a (V)	
10 1003		36 36 37	2 3 3	9 10	00 28	-24		-	'		'	'			25	A-4a (V)	
-K BORIN			2 3 4 ,	10	SS OC	-25	,	0	38	38	24	27	19	ω	34	A-4a (5)	
DI 99 2-2-		30	2 3 3	9 10	00	-26			1	1	'	,	ı		26	A-4a (V)	
HARD, GRAY AND BROWN, SANDY SILT, SOME GRAVEL, LITTLE CLAY, DAMP	587.2		6 6 36 36	31 7	8 8	-27		'	1	1	'	ı	ı	ı	ω	A-4a (V)	
5/.91 (1/10)	584.7	- 42	30 23 40 5	91 10	00 S	-28	- 1	3	56	29	15	21	16	5	6	A-4a (2)	
DENSE TO VERY DENSE, RED BROWN AND GRAY, GRAVEL AND STONE FRAGMENTS WITH SAND AND SILT, LITTLE CLAY, DAMP			7 18 24	51 4	4 S0	-29	,	<u> </u>	'	1	'	,	,	1	4	A-2-4 (V)	
NNORKI	844-0 8-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0-0-0 2-0 2		16 21 30	74 8	с С	30	ლ	5	2 25	21	9	20	17	e	10	A-2-4 (0)	
CLAYSTONE, GRAY AND RED-BROWN, SEVERELY	<u>17</u>	 ≤	6 50/5"	~~~~	2 S0	-31	,		-	•	'	<u> </u>	•	,	17	Rock (V)	

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	CLAYSTONE, GRAY AND RED-BROWN, SEVERELY WEATHERED VERY WEAK, ARENACEOUS									SILTSTONE, GRAY, HIGHLY WEATHERED, VERY WEAK	TO WEAK, THIN BEDDED, ARGILLACEOUS,	CALCAREOUS, JOINT DISCONTINUITIES, FRACTURED TO		SUCKENSIDED, BLOCKY, FAIR TO POOR SURFACE CONDITIONS; RQD 69%, REC 100%.	SILTSTONE, GRAY, MODERATELY WEATHERED,	SLIGHTLY STRONG, MEDIUM BEDDED, ARENACEOUS,		ROUGH, FAIR SURFACE CONDITIONS; RQD 76%, REC	100%.	



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	2/20	ERB	ΡL										
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	END:	(%)	с Г										
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	ART:	ц Ю	Яß										
	_ st	ЧН	(tsf)										
	39, 62' RT.	SAMPLE	□					7-7201					
	400+3	REC	(%)				001	2					
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	OFFS	SPT/	RQD				aa	8					
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	SF			3': con	5': con	9': qu	GRAY	TRON	JS, JC	D TO F	R SUF	(panu	
	08676			56	57.	- 58.		TLY S	REOL	'UREL	H, FAI	(contir	2
) 1			<u> </u> 356.2	<u> 9</u> 57.3	ฏ 58.5	SILTSI	SLIGH'	CALCA	FRACT	ROUG	.%00	1 1 9 6
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	CL W CL W CHECKED DMV	
TITIES: PLACED BENTONITE CHIPS, PUMPED 25 LB. BENTONITE POWDER, 94 LB. CEMENT, 50 GAL. WATER	SOIL PROFILE - LANDSLIDE BORING LOG B-002-1-20	
OTES: NONE BANDONMENT METHODS, MATERIALS, QUAN	MOE-7-7.55	
05/8/51 - ТОЭ.ТОД НО - (71 X 11) - ОО ВОЛИВ ОО ВО ТОО ОВАДИАТС	12/34	

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1 OF 1	HOLE																			
.4 π. 11	ODOT CLASS (GI)		A-7-6 (16)	A-7-5 (17)	A-4a (V)	A-4a (V)	A-4a (6)		A-4a (V)	A-6a (9)	A-2-4 (V)	A-2-4 (0)		A-1-b (0)	A-2-4 (0)	A-2-4 (0)	A-2-4 (V)	A-2-4 (0)	CORE	CORE
9306	MC		15	25	29	25	23		25	28	18	23		14	12	13	ი	13		
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<u>16568</u>	ERBE		27	31		,	21		•	21		d N		ď	17	16		17		
39.6	ATT LL		51	56			29		•	33		Ę		Ч	50	20		20		
610.	ठ (२)		51	55			51		•	30		12		9	13	9		9		
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86.	т Ъ		2.5	4.0		0.5	0.7			'	'									
MIE: (%):	SAMPL		SS-1	SS-2	SS-3	SS-4	ST-5		SS-6	SS-7	8-SS	6-SS		SS-10	SS-11	SS-12	SS-13	SS-14	NQ2-1	NQ2-2
	REC (%)		78	94	100	100	100		100	100	89	00		67	83	100	100	100	26	100
I KAII	N ₆₀		4	10	ور	10			Q	0	2	16		27	42	52	85			
	μđ		5 5	4 0	5	6 4			5	0	" 7	~ ~	4	8	3 14 15	7 8	30	0/5"	37	20
	ഗല	1		N	ε	N					4	2 2					1			
2 2	SH	-	3 7	4 0	9 /								3 1	й К _	5 5			8 8		$\begin{array}{c c} + & + \\ + & +$
HSA / NC	DEP1									W 594.1									— ТR —	C L
3.25 SPT	ELEV. 610.1		606 6	0.000	604.1					594.1	591.6		586.6		584.1	581.6	578.1	575 7	572.7	565.7
					<u> </u>								$\mathcal{O}_{0}^{\mathcal{O}}$							
이 되고 이 되고 이 되고															•=====					<u></u>
LING ME		SILT,		CLAY, AMP	T, SOME					SAND,	sray, Nd and			. AND/OR TRACE	D/OR TLE) GRAY, ND AND		٠, ۲,	DDED, NS; RQD), VERY ROW, ROW, NNING; NACEOUS	ERATELY shtly 3%, REC
SAM	N	"UND"		ASTIC VD, D,	Y SIL					OME	ACE O			SILT,	T, LIT	N ANI		С С Ц	DITIC NAR NAR NAR NST, NAR NST, ROI	
	RIPTI	¥,"		SAN	SAND					X, S(S WI			, GF TLE	D SIL	S WI	OR S.		CONTRACTION CONTRA	AS: R
11/20		, DAN		RACE	NN, S					CLA	ROWP			GRA) D, LIT	D ANI	NENT NENT	AND/	L.	PEN, ACE, NSHE, NSHE RVED	
8/		AND		EL, T	BRO					t ane	E, BF			AND SAN	SAN	RAGI	VEL	y, dai	Y WI O STI SURF SURF FR/ FR/ SBSEI BSEI BSEI BSEI BSEI	CON ZED, COND Psi eam
ġ	, HER	, BR CES		, RE RAVI	, FF,					, SILI		ī		WNN	MITH	NE NE NE NE	GRA	d gra	IIGHL NG TORE TURE AMIN, ARIN GGHL, ARIN VS, O VS, O VS, O VS, O	DISC CTUF CE C CE C (495)
	Ŷ	STIFI , TRA		STIFI CE G	0 S1					NMC	UM E			, BRC NTS /	NUC NIC	TO [8 STO \Y, W	RAY,	M an		JRFA JRFA u = 6 indstc
11/20		ERY VEL		TRA						BR	DOR DOR			GME	C-BR	INSE DOR	ы, O	brov	GRA GRA GRA GRA COND COND COND COND COND COND COND COND	US, J D TO IR SL 1': sa
/98U		TO VI GRA		TO VI SILT,	M ST MOIS					SOFT	L AN			M DE Wet	E, RE Wet	M DE ITTLE	DENS	35'	ATE CONE SATE CANE CANE ONE SATE ONE	.REO H, FA 43.
ART.		RIFF -		ND";	EDIU LAY, i					ERY (20SE	」 		EDIU TONE	ENSE LAY, J	EDIU RAVE LT, L	ERYL	AMP 33.5	MESI MESI MESI MESI MESI MESI MESI MESI	ALCA RACT OUGI 00/GI 00%. 143.7
		io 亡		lio ≤	120					15 Š		5		∣ ≥່ທ Ω		500		E 12 (2)	่ II รรดชเช ต รุธธิชชชช	いほうにのの

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	DRAWN CLW CHECKED DMV	
TITLES: PUMPED 12.5 LB. BENTONITE POWDER: 94 LB. CEMENT: 50 GAL. WATER	SOIL PROFILE - LANDSLIDE BORING LOG B-002-2-20	
TES: NONE ANDONMENT METHODS, MATERIALS, QUAN	MOE-7-7.55	
25/8//Sr - ТОЭ.ТОО НО - (71 X 11) ЭОЈ ВИЛОВ ЛОО ТОО ОЯАФИАТР	13/34	

PROJECT: MOE-7-07.55 DRIL	LLING FIRM / OPERA	TOR: DHDC / A.	Ŀ.	DRILL	RIGM	OBILE B-	-57 TRA	OK R	3 STA	TION	/ OFF	SET:	401	+46, 1	0' RT.		ATION ID		
TYPE: LANDSLIDE SAM PID: 108676 SFN: DRIL START: 7/27/18 FND: 8/1/18 SAM	APLING FIRM / LOGG LLING METHOD: API ING MFTHOD [.]	ER: HDR / S. RE 3.25" HSA / NQ SPT/ST/NO	ED	CALIB FNFR(er: <u>a</u> Ratio 3y ra	<u>UTOMA</u> N DATE: TIO (%)	TIC HAN : 82	<u>118</u>	ALK	GNME VATIO	μωι Έχχις	33.1 (SF MSL) 9.6169	EOE EOE	30 930	34.5 ft. 476	-0-10 PAGE 1 OF 2		
MATERIAL DESCRIPTION		ELEV. DEPTHS				EC SAM			GRA		N (%)			BER		ODOT CLASS (GI)			
ASPHALT PAVEMENT (8.5 INCHES) CONCRETE PAVEMENT (5 INCHES)		643.1 642.4 642.0				(%)	۳ -	ei ei	3	2	ō	5	т 		2				
STIFF, BROWNISH-GRAY, CLAY, SOME SILT, I SAND, LITTLE GRAVEL, MOIST (FILL)			- 2 -								2	12	с С		ő	A 7 6 (12)			
				0 0 4	2 9				-	D	74	}	й ?	- +	R 13	(71) 0-7-Y	- 1		
MEDIUM STIFF TO STIFF, REDDISH-BROWN, 1	TRACE	638.1	- 2 - 7	ω 4	6 6	20	2-2	. 75 -	·		1			'	26	A-7-6 (V)			
GRAY AND TAN, SILT AND CLAY , SOME SAND GRAVEL, SEVERAL SANDSTONE AND SHALE FRAGMENTS THROUGHOUT, DAMP			- 6 <u>-</u> - 7 <u>-</u> 3	N	٥	90 20	5.3 2.1	- 0.	•	•	•			'		A-6a (V)			
			~ 8	3 5	7	61 St	2 	. 75 -	•	•		•		'	22	A-6a (V)	I		
		<u> </u>	6 -	310	8	26 S	5-5 1.	20	•	•	•				21	A-6a (V)			
@ 9.5' - 10.3' : pushed tube only 8 inches due to l fragment at the bottom. The tube was damaged a	and rejected.		- 10 - 10			94 S	မ	<u>' </u>	· -	•	•	•		<u>'</u>	•	A-6a (V)			
		<u> </u>	- 11 - 12 - 12 - 12 - 12 - 12 - 12 - 12	4 6	4 4	11 St	2-2	'	•	•				'	9	A-6a (V)			
			- 13 -	6 5	15 (56 SS	9-8	25 -	•	•	ı			'	16	A-6a (V)			
			- 14	6 4	4	72 SS	3-9 2.(00	1	,	ı	,		'	9	A-6a (V)	Γ		
			- 15	56	15	39 SS	-10	75 -	·	'	1			'	18	A-6a (V)	I		
			- 17 - 3	5 7	17 (37 SS.	-11 2.(00	·					<u>'</u>	20	A-6a (V)	1		
@ 18.0' - 20.0' : Qu = 1445 psf			- 10 - 10			33 ST.	-12 2.0	00 16	3 22	4	22	36	35 2	1 14	19	A-6a (6)	1		
			- 20 + 2	2 2	1	78 SS	-13 2.(8	<u> </u>	· ·		· ·		<u>'</u>	20	A-6a (V)	_		
			- 22 +	, <u>,</u>	39	S: 00	-14 3.5	20	·	,				'	9	A-6a (V)	-		
			- 23 -1	12 0 21	32 1		-15 2.5	50		, ,		· ·			19	A-6a (V)	-1		
@ 24.5' - 26.0' : obtained sample from auger spoil	oils (no SPT		- 24	2 ⁻			2 4	8					·		2 5				
recovery)			- 26	<u>ດ</u>	2	<u>}</u>		' .	·					'	-	(v) 80-7			
GPJ			- 27	8 9	19	11 SS	-17 2.	8	•	•	•	•		·	17	A-6a (V)			
9 [°] (SNOIL)			- 28 -	812	28 1	SS 00	-18 2.	50 -	•	•	1			'	16	A-6a (V)			
аказа			- 30 - 1	0 8 21	40	00	-19 2.	20	•	•				•	9	A-6a (V)			
d dətaq			- 31 - 5	5 15 9	33 1	oo ss	-20	50 -	,		I				19	A-6a (V)			
udu) Se		6.609	- 32 - 6 - 33 - 6	11	29	SS-0	21A 3.	' 00 G	·	•	•	•		'	8	A-6a (V)			
STIFF TO VERY STIFF, MOTTLED BROWN ANI BILT AND CLAY, LITTLE SAND, MOIST	VD GRAY,	<u> </u>	- 34 4	2 L L	22 1	SS 00	-22 2.(· ·	ı <u> </u>	· ·			<u> </u>	25	A-6a (V) A-6a (V)	<u>,</u>		
		909	- 35 - 5 - 36 - 5	2 2	19	00 SS	-23 2.	50 -	· ·	·	1			' .	24	A-6a (V)	I		
MEDIUM STIFF, MOTTLED BROWN AND GRAY AND CLAY, TRACE SAND, MOIST (@ 36.5' - 38.5' : Qu = 2660 psf	Y, SILT		- 37			38 ST.	-24 1.	25 0	0	4	49	37	<u>2</u>	8	23	A-6a (9)	I		
- - -			- 39 - 39	4	4	S. S.	-25 0.7	75 -		·		· ·		<u>'</u>	24	A-6a (V)			
062081020			- 40	ຍ ຕ	~ ~	00 SS	-26 0.5	20	·					'	24	A-6a (V)			
95067910			- 42	5 3	8	00	-27 0.5	50 0	0	ø	55	37	31	9	25	A-6a (9)	_		
//01243/			- 43	4 7	4	S S O	-28 0.	75 -	<u> </u>	· ·		· ·		'	23	A-6a (V)			
ловкиле			- 45 -		4	S S O	-29 0.5	50 -	· ·	'	ı		· ·	'	26	A-6a (V)			
- C:/PVW			- 46	5 3	9	S S O	-30 0.4	50	· ·				· ·	'	24	A-6a (V)	-		
92:60 02/			- 48	0 K	9	S: 00	-31 0.	50	<u> </u>	· ·	1	· ·		<u>'</u>	25	A-6a (V)	1		
31/21 - 10			- 49 -	э Э	7	S 00	-32 0.1	75 0	0	∞	54	38	30	9	27	A-6a (8)	-		
19.100 I		591.6 W 591.6	- 51 - 2	3 10	~ ~	00 SS-	33A 1.	25 -	<u> </u>	·		·		<u>'</u>	27	A-6a (V)	-1		
Device the constant of the con	ורע, נודדנב		- 52 0	<u>ه</u> 0		-SS SS	33B -34		· ·					· ·	- 27	A-4a (V) A-4a (V)	I I		
X (1) 50			- 53 -	9	. 8		-35		2	49	25	18	Z	Ž	i 74	A-4a (2)			
זאואפר			- 55 44	~ .	2 :		<u>}</u>		,	:	ł	2	;	<u>:</u>	i ĉ				
HARD, GRAY, SILT AND CLAY, LITTLE SAND, E	DAMP	586.6	- 56 + 6	4 6 6	- <u>-</u>		37 10		· ·	, É	- 4	- «	· 2	· ÷	²⁰	A-43 (V)			
000 02			- 58 +	4141	5	2			v	2	,	2 2	- -	-	-	A-04 (0)			
HADINAT 2		583.6 TR	- 59	45 50/3" 7	•	73 SS	-38 4.	20	· -	•				<u>'</u>	13	A-6a (V)			
										-			-						
							<u>ц</u>	-				u							DRAWN
WOE-7-7.55			Ē				i S C	с	č	03- - -	ģ	12 12						U	CLW

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3-0-18	HOLE											
F2 B-00	ODOT CLASS (GI)	Rock (V)	Rock (V)	Rock (V)	Rock (V)	CORE	CORE	CORE		CORE	CORE	CORE
G 2 OI	NC NC	14	10	ი	4							
٦ ا	SERG PI	ŀ	•									
8/1/18			•	•								
ENC.	l (%) si cL	- -	•	·								
27/18		 .	•	-								
T: 7/2	GRAD	ŀ	•									
STAR	GR _	- 	•	· ·								
<u>.</u> .	E Hr (tst		'		'							
, 10' F	SAMPL ID	<u>SS-3</u>	SS-40	SS-4	SS-42	Ra-1-	NQ-2	NQ-3		NQ-4	NQ-5	NQ-6
t01+46	REC (%)	45	89	100	100	100	100	100		100	100	100
` 	Neo		•		•							
OFFSE	SPT/ RQD	50/5"	29 50/3"	50/5"	50/4"	12	0	22		54	0	15
ON /			<u>6</u>	83 63 E	64	65 - 66 - 67 -	89	70 + 71 + 72 +	73 -	75	8	8 82 84
STAT	DEPTHS								_			
20	≣V. 31.	-			8.6	9				0	3.1	۔ ب ی
-//-=	ELF 58,				22 21	3 1193	der tig tige		: জনসন্ধ		28	<mark>یں ب</mark> ے ایک ایک
MOE												<u>enalettette</u> -
PROJECT:	PTION	E GRAY, HIGHLY				WEATHERED, RAINED, THIN Y ROUGH TO TAL BEDDING 2D 14%, REC 100%.	RATELY VE GRAINED, SSIONAL VERY CTURED WITH	CES ALONG %.		ERED, WEAK TO NED, LAMINATED RED WITH CES ALONG FRAL INTERBEDDEI 20%.	ERED, SLIGHTLY	ED, LAMINATED TO BEDDING IN TINGS, TO ROUGH DING JOINTS; RQD
): 108676 SFN:	MATERIAL DESCRI AND NOTES	CLAYSTONE, REDDISH-BROWN, TRAC	WEALHERED, VERT WEAN. (Continued			SHALE, GRAY, SEVERELY TO HIGHLY VERY WEAK TO WEAK, VERY FINE GI BEDDED, FRACTURED WITH SLIGHTL ROUGH SURFACES ALONG HORIZON JOINTS, NUMEROUS CLAY SEAMS; RU	SILTSTONE, GRAY, HIGHLY TO MODE WEATHERED, SLIGHTLY STRONG, FII LAMINATED TO THIN BEDDED, OCCA WEAK TO WEAK SHALE SEAMS, FRAG	SLIGHTLY ROUGH TO ROUGH SURFA BEDDING JOINTS; RQD 14%, REC 100 @ 69.3' - 69.5' : vertical fracture @ 71.2' - 71.3' : vertical fracture	@ 72.5' - 72.8' : vertical fracture	SHALE , GRAY, MODERATELY WEATH SLIGHTLY STRONG, VERY FINE GRAII TO THIN BEDDED, SLIGHTLY FRACTU SLIGHTLY ROUGH TO ROUGH SURFA HORIZONTAL BEDDING JOINTS, SEVE SILTSTONE SEAMS; ROD 54%, REC 11 @ 75.9': Qu = 2255 psi	SILTSTONE, GRAY, SLIGHTLY WEATH	STRONG, VERY FINE TO FINE GRAINI THIN BEDDED, OCCASIONAL CROSS- SILTSTONE, OCCASIONAL SHALE PAI FRACTURED WITH SLIGHTLY ROUGH SURFACES ALONG HORIZONTAL BEC 43%, REC 100%.

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	DRAWN CLW CHECKED DMV
NITIES. BACKFILLD WITH 4 BAGS BENTONTE GROUT, MARD 97 GAL, WATER	SOIL PROFILE - LANDSLIDE BORING LOG B-003-0-18
IES. NONE MATERIALS, QL	MOE - 7 - 7 .55
	15/34

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Mutable Constrained ELP ((C) (C) (C) (C) (C) (C) (C) (C) (C) (C)	TTPE: LANUSCIDE SAMIFLING FIRM/ PID: 108676 SFN: DRILLING METHO START: 7/26/18 END: 7/26/18 SAMPLING METHO	LUGGEN. <u>ПUR/ 3. КЕ</u> D: <u>3.25" HSA / NQ</u> DD: SPT/ST/NQ		RATION GY RATI	DATE: 0 (%):	<u>2/27</u> 82.	7 7			z Z Ö	343.2	0 (MSL	016, FO	B: -80.93	90.5 ft 0180	
Bit Not were inversion 663 1 <th>MATERIAL DESCRIPTION AND NOTES</th> <th>ELEV. DEPTH: 643.2</th> <th>S SPT/ RQD</th> <th>N₆₀ RE (%</th> <th>C SAMF</th> <th>LE HI (ts</th> <th>f) GF</th> <th>GRAI cs</th> <th>DATIC</th> <th>N (%</th> <th>CL /</th> <th></th> <th>RBER L</th> <th>× ح</th> <th>C CLA</th> <th>DOT SS (GI)</th>	MATERIAL DESCRIPTION AND NOTES	ELEV. DEPTH: 643.2	S SPT/ RQD	N ₆₀ RE (%	C SAMF	LE HI (ts	f) GF	GRAI cs	DATIC	N (%	CL /		RBER L	× ح	C CLA	DOT SS (GI)
TITP: TO VERY FIFTE REDORGATEDON: TRACE 6827 11 1 <th1< th=""> 1 <th1< th=""> 1</th1<></th1<>	ASPHALT PAVEMENT (10 INCHES) SONCRETE PAVEMENT (6 INCHES) STIFF, BROWN, SILT AND CLAY , "AND" SAND, LITTLE SRAVEL, MOIST (FILL)	642.4		11	s S	· ·		24	12	22	30	36	1	7	9-4 9	ìa (4)
11 (F · 10F · 0.0 - 4100 art b 0.0 · 10 · 10F · 10F · 10F · 0.0 · 10 · 10 · 10 · 10 · 10 · 10 · 1	TIFF TO VERY STIFF, REDDISH-BROWN, TRACE SRAY, SILT AND CLAY , SOME SAND, TRACE GRAVEL, NOIST	239.7 239.7	5 4 3 2 2 2 2	6	s, s	4		· ·							4 P	ia (V)
11 (P - 132): cock at the diameter ST-5 (signity dramages buildown) 100 - 130; cock at the diameter ST-5 (signity dramages buildown) 100 - 130; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages buildown) 100 - 100; cock at the diameter ST-5 (signity dramages BUE ST-5 (sock at the diameter ST-5 (signity dramages BUE ST-5 (sock at the diameter ST-5 (sock at the diame				11 44	S:	3.2.5	- ' 0	•							2-6	ia (V)
Fig. 5: 10: 10. red at 100 pcf 10: 11. 0: -120: .0c4 at 10: 120: .0c4 at 10: 120 pcf 10: 120: .0c6 at 10: 00 pcf 10: 120: .0c6 at 10: .0c6 at 10: 00 pcf 10: 120: .0c6 at 10: 00 pcf 10: 120: .0c6 at 10: .0			- 9 - 3 - 10 - 3 5 6	15 56	SS-	4 2.5								~	5 A-6	ia (V)
Fig. 5: 10: 17: 10: 17: 10: 10: 17: 10: 10: 10: 10: 10: 10: 10: 10: 10: 10	: 11.0' - 13.0' : Qu = 4100 psf : 11.0' - 13.0' : rock at tip of sample ST-5 (slightly damaged bottom)		11	75	ST-	- ' 	00	17	œ	24	43	35	7	- -	2-A-(ia (8)
Fig. 2 5 1 6 5 1 6 1 <td></td> <td></td> <td>- 13 5 - 14 6 8</td> <td>19 67</td> <td>-SS</td> <td>9.5</td> <td>- 03</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td>8 A-6</td> <td>ia (V)</td>			- 13 5 - 14 6 8	19 67	-SS	9.5	- 03							-	8 A-6	ia (V)
First Fieldown AND TAN SLT AND CLAY. SOME 633 19 7 534 225 -			- 15 - 16 - 16 - 16 - 16 - 16 - 16 - 16	18 67	s s	7 3.2	- 55	·		ı				-	8 A-6	ia (V)
THY STIFF BROWN AND TAN BILT AND CLAY. SOME 20 4 23 72 559 275 2		623.7 623.7	- 18 - 18 - 18 - 19 - 19 - 19 - 19 - 19	19 72	S	5.2	-	-		ı				-	8 -6	ia (V)
27 2 4 6 22 78 55-10 350 2 19 5 32 42 3 27 2 7 8 1 15 5 15 5 2 7 2 1 17 5 1 5 32 42 3 27 2 7 8 10 55-11 175 5 1 5 32 42 3 46 3	ERY STIFF, BROWN AND TAN, SILT AND CLAY , SOME ND, TRACE GRAVEL, MOIST		- 20 - 4 - 21 - 4 9 8	23 72	s.	9 2.7		-		ı	· ·			-	9-6	ia (V)
Trip Trip <th< td=""><td></td><td></td><td>- 22 - 23 - 4 6 - 24 - 10</td><td>22 78</td><td>SS .</td><td>3.5</td><td>00</td><td>19</td><td>2</td><td>32</td><td>42</td><td>35 2</td><td>7</td><td>- -</td><td>8</td><td>ia (9)</td></th<>			- 22 - 23 - 4 6 - 24 - 10	22 78	SS .	3.5	00	19	2	32	42	35 2	7	- -	8	ia (9)
RY STIFF. REDDISHBROWN, TRACE GRAY. SLT 0102 27 4 2			- 25 7 7 - 26 7 7 8	21 67	S.	1.1	- 22	'						-	2 A-6	ia (V)
FIF: REDDISH-BROWN TO GRAY. SILTY CLAY. 610.2 33 7 21 100 SS-14A 2.50 -	RY STIFF, REDDISH-BROWN, TRACE GRAY, SILT ID CLAY, LITTLE SAND, MOIST	7.00	- 27 - - 28 4 8 - 29 11	26 10	-SS	3.2	5 0	ര	~	38	46	37	7	4	0 -A-6	a (10)
32 32 7 21 100 SS-14A 250 - <			- 30 - 6 - 31 - 6 9 - 31 - 9	25 10	SS-1	3 3.0	- ' 0	'		1				7	1 A-6	ia (V)
35 4 7 21 100 SS-15 2.25 0 1 1 52 46 3 EDDISH-BROWN WITH TRACE 665.7 -36 -37 - 1 100 SS-15 2.25 0 1 1 52 46 3 EDDISH-BROWN WITH TRACE -38 -3 -12 100 SS-16 0.75 0 1 1 52 46 3 ND, MOIST -42.0°: Attempted undisturbed shelby tube, but was not recovered. An SPT sample was then driven mple was not recovered. An SPT sample was then driven 601.2 -41 -41 -41 -41 -41 -41 -7 <t< td=""><td>IFF, REDDISH-BROWN TO GRAY, SILTY CLAY, XACE SAND, MOIST</td><td>610.2</td><td>- 32 - 33 - 7 7 - 34 - 8</td><td>21 10</td><td>0 SS-1</td><td>4A 2.5 4B 1.5</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>5</td><td>A-6</td><td>ia (V) b (V)</td></t<>	IFF, REDDISH-BROWN TO GRAY, SILTY CLAY , XACE SAND, MOIST	610.2	- 32 - 33 - 7 7 - 34 - 8	21 10	0 SS-1	4A 2.5 4B 1.5								5	A-6	ia (V) b (V)
EDIUM STIFF, GRAYISH-BROWN WITH TRACE 605,7 37 1 10 5 12 100 55-16 0.75 0 0 11 50 39 3 CDDISH-BROWN AND GRAY, SILT AND CLAY, LITTLE 38 3 4 12 100 55-16 0.75 0 0 11 50 39 3 A0.0 ⁺ - 42.0 ⁺ Attempted undisturbed shelby tube, but 94 5 12 100 55-16 0 0 11 50 39 3 A0.0 ⁺ - 41.5 ⁺ . mple was not recovered. An SPT sample was then driven 601.2 94 57-17 0.75 -			- 35 - 4 - 36 - 4 - 36 - 7 8	21 10	SS-	5 2.2	55	-	-	52	46	37		0	3 A-6	b (10)
40.0° - 42.0°: Attempted undisturbed shelby tube, but mple was not recovered. An SPT sample was then driven m 40.0° to 41.5'. 94 ST-17 0.75 -	EDIUM STIFF, GRAYISH-BROWN WITH TRACE EDDISH-BROWN AND GRAY, SILT AND CLAY , LITTLE ND, MOIST	605.7	- 37 - 38	12	- SS	6 0.7	20	0	7	50	39	32		5	3 4-6	la (9)
EDIUM STIFF, BROWNISH-GRAY, SANDY SILT, SOME 42 43 10 100 55-18 0.75 -	40.0' - 42.0' : Attempted undisturbed shelby tube, but mple was not recovered. An SPT sample was then driven m 40.0' to 41.5'.	60 12 12	40	94	ST-1	2 0.7		•		ı					9-6	ia (V)
45° : occasional thin wet sifty sand seam	EDIUM STIFF, BROWNISH-GRAY, SANDY SILT , SOME AY, MOIST		- 42 43 43 43 43 43 43 43	10	0 SS-1	8 0.7	. 5							N .	4 A-2	a (V)
	45': occasional thin wet silty sand seam		- 45 - 45 - 46 - 2	7 10	SS-0	6 0.7	5 0	0	59	46	25	24	<u></u>	۰۵ ۱۹	3	la (7)

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STANDARD ODOT SOIL BORING LOG (11 X 17) - OH DOT.GDT - 12/18/20 09:26 - C:/PWW

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DMV

B-004-0-18

PID: 108676 SFN:	PROJECT: M	OE-7-07.5	5 STA ⁻	TION / C	DFFSET	4	2+45, 1	0' RT.	ST	ART: 	7/26/1	□ ∞	ġ	7/26/1		PG 2 0	DF 2 B-	004-0-18
MATERIAL DESCRIF AND NOTES	NOIL	ELE' 583.	V. DEPTHS	0) E	SPT/ RQD	N ₆₀ RI	EC SAI %)	MPLE	HP (tsf)	GR OR	ADATI S FS	ON (%	c)			S S	ODOT CLASS (G	
DENSE, OLIVE-BROWN AND GRAY, GF SAND AND SILT, TRACE CLAY, MOIST 7 (continued)	TO WET			61	1 23 25	66 7	ω Ň	S-25		23 2	1 21	25	10	d N	Ľ	16	A-2-4 ((
HARD, REDDISH-BROWN, TRACE GRA SOME CLAY, DAMP	.Y, SANDY SILT,			63 1 64 63	5 35 48	14 7	2	S-26	4.50	0	3 16	36	35	59		17	A-4a (7	
				65 <u>1</u> . 66 <u>1</u>	5 34 1 47	12 8	23 23	S-27	4.50		· ·			-		17	A-4a (V	
SHALE, BLUISH-GRAY, HIGHLY WEATH	HERED, WEAK.	575. 575.	7 	<u>6</u> 88 9. 9. 9. 9. 9. 9. 10. 11. 11. 11. 11. 11. 11. 11. 11. 11	0/3"		8	S-28			' }	-] 			'!	16	Rock	
SHALE, GRAY, MODERATELY TO HIGH VERY WEAK TO WEAK, VERY FINE GF LAMINATED TO THIN BEDDED, OCCAS CALCAREOUS SEAMS, OCCASIONAL C MODERATELY FRACTURED WITH SLIG SURFACES ALONG HORIZONTAL BEDI 32%, REC 96%.	LY WEATHERED, AINNED, IONNED, SIONAL CLAY SEAMS, BHTLY ROUGH DING JOINTS; ROD	<u>572</u>	<u> </u>	72	28			Q-1					 			4	CORE CORE	
		; 		75 ++ 76 ++ 77 +5 77 +5 77 +5 77 +5 77 +5 78 +5 79 +5 79 +5 70 +5	90	0	Z	0-2									CORE	
SILTSTONE, GRAY, MODERATELY WE, TO SLIGHTLY STRONG, VERY FINE GF MEDIUM BEDDED, OCCASIONAL THIN MODERATELY FRACTURED WITH ROL ROD 19%, REC 94%. @ 83.5' - 83.5' cou = 33 psi (very weak (@ 83.5' - 83.7' : high angle fracture	ATHERED, WEAK ANNED, THIN TO SHALE PARTINGS, JGH SURFACES; seam)			82 83 84 85	50	0,	24 Z	0 3									CORE	
SHALE, GRAY, HIGHLY WEATHERED. WEAK, VERY FINE GRAINED, THIN BEI INTERBEDDED ZONES OF SLIGHTLY S CALCAREOUS SHALE, FRACTURED TC	VERY WEAK TO DDED, STRONG MODERATELY		<u></u>	- 86	27	-	2 0	Q-4									CORE	
FRACTURED WITH SLIGHTLY ROUGH HORIZONTAL BEDDING JOINTS; RQD '	SURFACES ALONG 15%, REC 100%.	552			0	-	2 0	Q-5									CORE	

09:26 - C. PVMORKING/EAST01/D1679058/20180730_MOE-7-7.28_2017 10-AA_TYPED LOGS (UPDATED DESCRIPTIONS).GPJ

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			DRAWN CLW CHECKED	DMV
		NTITIES: BACKFILLED WITH 3.5 BAGS BENTONITE GROUT; MIXED 80 GAL. WATER	SOIL PROFILE - LANDSLIDE PODING LOG P-001-0-18	
	IOTES: NONE	ABANDONMENT METHODS, MATERIALS, QUA	MOE - 7 - 7 .55	
02/81/21 - TGD.TOD HO - (71 X 11) ОС ВОГІВОТ ОО БОТІВОТ О	Z	4	17/3	4

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							DRAWN CLW	CHECKED DMV
СОЗ					TER		- ANDSLIDE	-004-1-20
− 46 − − 47 − − 47 − − 48 − 563.5 − − 49 − − 49 − − 49 − − 49 − − 49 − 563.5 − − 49 − − 59 − − 49 − − 50 − − 50 − − 51.2					ED 25 LB. BENTONITE POWDER; 94 LB. CEMENT; 50 GAL. WA		SOIL PROFILE - 1	BORING LOG B
RED RED K BEDDED K BEDDED K BURFACE SUNFACE SUNFACE					UANTITIES: PUMP			
SILTSTONE, GRAY, MODERATELY WEATHE MODERATELY STRONG, MEDIUM TO THICH ARENACEOUS, JOINT DISCONTINUTIES, S ARENACTURED TO MODERATELY FRACTURE SLIGHTLY ROUGH INTACT, GOOD TO FAIR	CONDITIONS; ROD 89%, REC 100%. @ 50.7': grades to sandstone			NOTES. NONE	ABANDONMENT METHODS, MATERIALS, Q			/ MUE-/-/.50
ר בעניגע מאיז איז איז איז איז איז איז איז איז איז	HO - (21 X I	BORING LOG (1	אם סססד צסור	AGNAT2	1	J	18	/34

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PAGE 1 OF 1 HOLE SEALED							1 1			1 1						1	-	
3.0 ft. 79 CLASS (GI)		A-6b (V)	A-6b (10)	A-6b (V)		A-4a (V)	A-4a (5)	A-2-4 (0)	A-2-4 (V)	A-1-b (V)	A-1-b (0)	A-1-b (V)	A-1-b (0) A-1-b (0)	A-3a (0)	Rock (V)	CORE	CORE	CORE
9299 WC	2	39	32	27	i	25	25	11	5	12	16	ω	1 4	23	-1			
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с <u>603</u>	3	· ·	38			· ·	21	00	· ·		~		2 00	٥	· ·			
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LC LC	2	· ·	15	·		· ·	40	- 58			56		13 22	57	· ·			
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	5	· ·	5	2	,	· ·	0	4	· ·		эё Э	-	,4 0	4	'			
86.8 86.8 (tst	<u> </u>	'	0.2	1 2	!	'	'	•	'		-			· ·	· ·			
ATE:	2	SS-1	SS-2	SS-3		SS-4	SS-5	SS-6	SS-7	8 - - - - - - - - - - - - - - - - - - -	6-SS	SS-10	SS-11A SS-11B SS-11B	SS-12	SS-13	NQ2-1	NQ2-2	NQ2-3
REC (%)	(0)	83	100	001		100	100	72	83	68	72	83	83	83	100	100	78	95
N°° RA N°° R		0	0	0	,	4	4	23	32	42	29	23	88	35				
CALIE ENER SPT/ ROD		- 0 0	0		0	1 2 1	5 1 2	5 6 10	5 7 15	9 19 10	11 10 10	597	14 29 32	9 10 14	50/6"	74	40	85
HSA / NQ2 PT / NQ2 DEPTHS	_	- 7 - 7		1 00	v 594.5 - 8 -	• - - - - -	- 11 - 12 - 12			- 18 - - 19 - - 20 -	- 21 - 22 22	- 23 - - 24 - - 25 -	- 26 - - 27 -	28 28 29 29 29	- TR	33 34	- 35 - - 36 - - 37 -	
3.25" 3.25" SI ELEV.	603.U				594.5			589.5		284.5			576.0	D 574.5	572.0	270.0		• 260.7 • 560.7
ETHOD:	μ																<u>nik (dan k</u> :	
108676 SFN:	/ SOFT TO SOFT BROWNISH-GRAY SILTY CLAY	E SAND, TRACE GRAVEL, MOIST				, GRAYISH-BROWN, SANDY SILT , SOME CLAY,		UM DENSE TO DENSE, GRAYISH-BROWN, TEL AND/OR STONE FRAGMENTS WITH SAND AND TRACE CLAY, WET		UM DENSE TO DENSE, BROWN TO RED-BROWN, FEL AND/OR STONE FRAGMENTS WITH SAND , E SILT, TRACE CLAY, WET			' DENSE, BROWN TO RED-BROWN, GRAVEL OR STONE FRAGMENTS WITH SAND, LITTLE SILT, E CLAY, WET	(DENSE, GRAY, GRAVEL AND/OR STONE MENTS WITH SAND, LITTLE SILT, TRACE CLAY, 56. BROWN, COARSE AND FINE SAND, LITTLE	/el, litte silt, trace clay, wet .e, gray, severely weathered, very weak.	STONE, GRAY, MODERATELY WEATHERED, ERATELY STRONG, MEDIUM TO THICK BEDDED, MACFOLIS, JOINT DISCONTINUTTES MODERATEI	TURED TO FRACTURED, NARROW, SLIGHTLY SH, INTACT, GOOD SURFACE CONDITIONS; RQD REC 87%. 33.2": sandstone seam .2" - 35.7": qu = 6,872 psi	SSTONE, GRAY, SLIGHTLY TO MODERATELY

0 09:30 - C:\PWWORKING\EAST01\D1679058\MOE-7-7.55 10-K BORING LOGS.GPJ

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	CLW CHECKED DMV	
UTITIES: PUMPED 12.5.LB. BENTONITE POWDER: 94.LB. CEMENT: 50 GAL. WATER	SOIL PROFILE - LANDSLIDE BORING LOG B-004-2-20	
TES: NONE ANDONMENT METHODS, MATERIALS, QUAI	MOE-7-7.55	
	19/34	



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OHIO DEPARTMENT OF TRANSPORTION OFFICE OF GEOTECHNICAL ENGINEERING PROJECT MOE-7-7.55 OGE NUMBER MOE-7-7.55 SAMPLE IDENTIFICATION BORING ID: <u>B-003-0-18</u> STATION: NOT RECORDED 3 SPECIMEN FAILURE SKETCHES OR PHOTOGRAPHS

BEFORE FAILURE

AFTER FAILURE

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Unconfined Compression Test Results	CTL ENGINEERING, INC.
ASTM D 2166, D 5102	2860 Fisher Road Columbus, Ohio 43204
Sample ID: B-004-1-20, ST-8, 10.5'-12.5' Avg. Sample Height (in.): 5.79 Avg. Sample Diameter (in.): 2.88 Height-to-diameter ratio: 2.02 Ultimate Strength (ksf): 2.19 Shear Strength (Ksf): 1.09 Avg. Rate of Strain to Failure(%): 1.93 Strain at Failure (%): 11.70 Initial Dry Density (pcf): 97.57	Client: HDR Engineering, Inc. Project: MOE-7-7.55 (10-K) Location: Monroe County, Ohio Project No. 20050114COL Lab Code No. N/A Date Tested: 8/31/2020 Reviewed by: SM
Moisture Content (%): 27.1 (Obtained Post Shear) Visual Description: Brown Silt and Clay (A-6a), moist Degree of Saturation: NA Sensitivity: NA Failure Type: Diagonal Shear	ASTM D 4318 ASTM D 6913 LL: 36 Gravel (%): 0 Silt(%): 54 PL: 23 Sand(%): 11 Clay(%): 35
2.50 2.00 1.50 (5) set 0.50 0.50 0.00 0.00 2.0 4.0 6.0 8	POST SHEAR POST SHEAR 0.0 10.0 12.0 14.0 16.0
Stra	in (%)
ENGINEERI	NG Z

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DRAWN CLW	CHECKED
SOIL PROFILE - LANDSLIDE	COMPRESSIVE STRENGTH TEST RESULTS
MOE-7-7 55	
23	/ ₃₄

CONSOLIDATED UNDRAINED TRIA	XIAL TEST ON C	CTL ENGINEERING, INC.	
AASHTO T 29	7 & ASTM D47	67	2860 Fisher Road Columbus, Ohio 43204
			Client: HDR Engineering, Inc PID NO. 108676 Project: MOE-7-7.55 (Task 10-K) Location: Monroe County, Ohio
Sample Type	Shel	lby Tube	Project No. 20050114COL
Date Set-up:	8/25/2020	8/25/2020	County, Rt. & Sec.: MOE-7-7.55
Date Sheared:	9/1/2020	9/1/2020	Station & Offset: NA
Avg. Sample Height (in.):	5.7957	5.7757	Sample ID: B-002-1-20, ST-10, 13.5'-15.5'
Avg. Sample Diameter (in.):	2.8500	2.8500	Lab Code No. 20050114COL
Height-to-diameter ratio:	2.03	2.03	Reviewed by: SM
Wet Density (pcf):	125.2	124.6	,
Dry Density (pcf):	103.0	103.3	
Void Ratio:	0.636	0.631	
Specific Gravity (assumed):	2.7	2.7	
Moisture Content (%):	21.6	20.6	
Cross Sectional Area (ft^2):	0.044	0.044	
Volume (ft^3):	0.02	0.02	
Confining Pressure (psf):	1872	3744	POST SHEAR
Rate of Axial Strain (%/min):	0.2071	0.2078	1872 psf
Compressive Strength (psf):	4699	5708	→ /µ= - 5.51 (A ⁺) =
Minor Principal Stress at Failure (psf):	1872	3744	
Major Principal Stress at Failure (psf):	6571	9452	
Failure Criterian (%):	Deviator Stress	at 15% Axial Strain	
β: Γ	0.97	0.96	
Specimen Saturation:	Wet	Method	
-FE			POST SHEAR
			3744 psf
Grading (ASTM D422)			Biologica = # 1.4 (Ra) Pr-1.25 million 16 / 27 C
% Agg:		1	
% Sand.:		1	State of the second sec
% Silt:		52	
% Clay:		46	
Atterberg Limits (ASTM D 4318)			
L.L.:		50	
P.L.:		26	
P.I.:		24	
L			

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DRAMN CLW CHECKED DMV	
SOIL PROFILE - LANDSLIDE COMPRESSIVE STRENGTH TEST RESULTS	
MOE-7-7.55	
24/34	



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		DRAWN CLW CHECKED DMV
CONSOLIDATED UNDRAINED TRIAXIAL TEST ON COHESIVE SOILS AASHTO T 297 & ASTM D4767 Sample Type Date Set-up: Shelby Tube Date Set-up: $8/21/2020$ $8/21/2020$ $8/21/2020$ Avg. Sample Height (in.): $8/24/2020$ $8/24/2020$ $8/25/2020$ Avg. Sample Diameter (in.): 2.8750 2.8750 2.8750 Avg. Sample Diameter (in.): 2.8750 2.8750 2.8750 Height-to-diameter ratio: 2.03 2.03 2.02 Wet Density (pcf): 122.1 124.5 125.3 Dry Density (pcf): 122.1 0.75 2.75 Moisture Content (%): 24.3 24.9 24.3 Cross Sectional Area (ft^2): 0.045 0.045 0.045 Noisture Content (%): 24.3 24.9 24.3 Cross Sectional Area (ft^2): 0.045 0.025 0.2057 0.2069 Confining Pressure (psf): 1584 3312 6624 1528 Major Principal Stress at Failure (psf): 3998 6650	CTL ENGINEERING, INC. 2860 Fisher Road Columbus, Ohio 43204 Client: HDR Engineering, Inc PID NO. 108676 Project: NOE-7-7.55 (10-K) Location: Monroe County, Ohio Project No. 20050114COL County, Rt. & Sea:: MOE-7-7.55 Station and Offset: NA Sample ID: B-0021-120, ST-18, 26'-28' Lab Code No. NA Reviewed by: SM Sample ID: B-0021-120, ST-18, 26'-28' Ibid Sample ID: B-0021-120, ST-18, 26'-28' Lab Code No. NA Reviewed by: SM POST SHEAR 1584 psf Image:	SOIL PROFILE - LANDSLIDE COMPRESSIVE STRENGTH TEST RESULTS
Atterberg Limits (ASTM D 4318) L.L.: 29 P.L.: 19 P.I.: 10 Visual Description: Brown Sandy Silt (A-4a), Some Clay,	POST SHEAR 6624 psf	95° Z - Z - 30

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			DRAWN CHECKED DMV
CONSOLIDATED UNDRAINED T AASHTO T Sample Type Date Set-up: Date Sheared: Avg. Sample Height (in.): Avg. Sample Diameter (in.): Height-to-diameter ratio: Wet Density (pcf): Dry Density (pcf): Uoid Ratio: Specific Gravity (assumed): Moisture Content (%): Cross Sectional Area (ft^2): Volume (ft^3): Confining Pressure (psf): Rate of Axial Strain (%/min): Compressive Strength (psf): Minor Principal Stress at Failure (psf): Major Principal Stress at	Shelby Tube Shelby Tube $8/28/2020$ $8/28/2020$ $8/28/2020$ $9/1/2020$ $9/1/2020$ $9/1/2020$ $9/1/2020$ $9/1/2020$ $9/1/2020$ 5.7517 5.7943 5.7023 2.8750 2.8750 2.8750 2.00 2.02 1.98 126.9 124.5 126.6 103.0 101.2 102.7 0.666 0.696 0.671 2.75 2.75 2.75 23.2 23.1 23.2 0.045 0.045 0.045 0.02 0.02 0.02 720 1440 2880 0.2086 0.2071 0.2104 2805 3206 3901 720 1440 2880 3525 4646 6781 Deviator Stress @ 15 % Axial Strain 0.97 Wet Method 21	CTL ENGINEERING, INC. 2860 Fisher Road Columbus, Ohio 43204 Client: HDR Engineering, Inc PID NO. 108676 Project: MOE-7-7.55 (10-K) Location: Momoe County, Ohio Project No. 20050114COL County, Rt. & See:: MOE-7-7.55 Station & Offset: NA Sample ID: B-002-2-20, ST-5, 10-12' Lab Code No. NA Reviewed by: SM Small County POST SHEAR 1440 psf Image: County POST SHEAR 2880 psf Image: County	SOIL PROFILE - LANDSLIDE COMPRESSIVE STRENGTH TEST RESULTS
P.L.: P.I.: Visual Description:	21 8 Brown Sandy Silt (A-4a), Some Clay, Moist		28 34

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Project: MOE-7-7.55 **SR-7 Landslide Stabilization**

Unconfined Compressive Strength (ASTM D7012)





B-001-0-18; NQ-3; 71.3' - 71.8' Average Diameter: 1.819" Average Height: 4.143" Bulk Density: 154.2 pcf Unconfined Compressive Strength: 156 psi and 666 psi (see note below)

Note: L/D ratio ~ 2.3



B-002-0-18; NQ-2; 68.5'-68.9' Average Diameter: 1.797" Average Height: 3.651" Wet Density: 162.7 pcf Dry Density: 156.4 pcf Unconfined Compressive Strength: 860 psi

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MOE-7-7.55

PROJECT NAME: SR-7 Landslide Stabilization

Unconfined Compressive Strength (ASTM D7012)



Note: L/D ratio ~ 2.03

UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE - ASTM D 7012	ETL
	ENGINEERING 😤

PROJECT NO: 20050114COL DATE: 9/3/2020

INTACT ROCK CORE - ASTM D 70 Method C

BORING NUMBER	B-002-1-20	TOP DEPTH(FT)	58.5	BOTTOM DEPTH(FT)	58.9
SAMPLE NUMBER	R-1	DISTRICT	10	PID NO.	108676
COUNTY	MOE	ROUTE	7	SECTION	7.55 (10-K)

FORMATION	Pennsylvanian Age
DESCRIPTION	Siltstone, Gray, Moderately Weathered, Slightly Strong
MOISTURE CONDITION	As Received

MEASUREMENT LENGTH(INCHES) DIAMETER(INCH				
MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)	LENGTH/DIAMETER	2.1
1	4.115	1.982	CORRECTION FACTOR	1
2	4.114	1.993	AREA(IN ²)	3.1
3	4.111	1.982	MASS (GRAMS)	546.2
AVERAGE	4.113	1.986	UNIT WEIGHT(LBS/FT ³)	163.4





BORING NUMBER	B-002-2-20	TOP DEPTH(FT)	42.9	BOTTOM DEPTH(FT)	43.3
SAMPLE NUMBER	R-2	DISTRICT	10	PID NO.	108676
COUNTY	MOE	ROUTE	7	SECTION	7.55 (10-K)
FORMATION	Pennsylvanian Ag	je			
DESCRIPTION	Siltstone, Gray, N	loderately Weather	ed,Mod	erately Strong	
MOISTURE CONDITION	As Received				

MEASUREMENT LENGTH(INCHES) DIAMETER(IN 4.085 1.988 1 4.114 1.974 2 3 4.108 1.991





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Method C

CHES)	LENGTH/DIAMET	ER 2.1
	CORRECTION FACT	OR 1
	AREA(II	N ²) 3.1
	MASS (GRAN	1S) 545.4
	UNIT WEIGHT(LBS/F	T ³) 163.8



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2390 Advanced Business Center Drive Columbus, Ohio 43228 Office: 614.527.7656 ww.dhdcinc.com



<u>MOE-7-7.55</u>

PROJECT NAME: SR-7 Landslide Stabilization

Unconfined Compressive Strength (ASTM D7012)



B-003-0-18; NQ-4; 75.9'-76.5' Average Diameter: 1.796" Average Height: 3.779" Wet Density: 166.1 pcf Dry Density: 159.7 pcf Unconfined Compressive Strength: 2,255 psi

Note: L/D ratio ~ 2.1

Project Name:

MOE-7-7.55



B-004-0-18; NQ-3; 83.0'- 83.5' Average Diameter: 1.772" Average Height: 3.809" Wet Density: 164.3 pcf Dry Density: 157.9 pcf Unconfined Compressive Strength: 343 psi

Note: L/D ratio ~ 2.1

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Unconfined Compressive Strength (ASTM D7012)

RESULTS TEST - LANDSLIDE STRENGTH PROFILE SOIL COMPRESSIVE S ŝ **>**-2-MOE

UNIAXIAL COMPRESSIVE STRENGTH OF	
INTACT ROCK CORE - ASTM D 7012	
	FNGINFFRING 2

PROJECT NO: 20050114COL DATE: 9/3/2020

INTACT ROCK CORE - ASTM D Method C

BORING NUMBER	B-004-1-20	TOP DEPTH(FT)	44.1	BOTTOM DEPTH(FT)	44.5
SAMPLE NUMBER	R-1	DISTRICT	10	PID NO.	108676
COUNTY	MOE	ROUTE	7	SECTION	7.55 (10-K)

FORMATION	Pennsylvanian Age
DESCRIPTION	Siltstone, Gray, Highly Weathered, Moderatly Strong
MOISTURE CONDITION	As Received

MEASUREMENT LENGTH(INCHES) 1 4.085				
MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)	LENGTH/DIAMETER	2.1
1	4.085	1.988	CORRECTION FACTOR	1
2	4.085	1.980	AREA(IN ²)	3.1
3	4.085	1.986	MASS (GRAMS)	543.1
AVERAGE	4.085	1.985	UNIT WEIGHT(LBS/FT ³)	163.7





PROJECT NO: 20050114COL DATE: 9/3/2020

INTACT ROCK CORE - ASTM D 7012

BORING NUMBER	B-004-2-20	TOP DEPTH(FT)	35.2	BOTTOM DEPTH(FT)	35.7
SAMPLE NUMBER	R-2	DISTRICT	10	PID NO.	108676
COUNTY	MOE	ROUTE	7	SECTION	7.55 (10-K)
FORMATION	Pennsylvanian Ag	ge			
DESCRIPTION	Siltstone, Gray, N	Ioderately Weather	ed,Mod	leratly Strong	
MOISTURE CONDITION	As Received				

MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)	LENGTH/DIAMETER	2.1
1	4.105	1.993	CORRECTION FACTOR	1
2	4.102	1.989	AREA(IN ²)	3.1
3	4.101	1.978	MASS (GRAMS)	546.0
AVERAGE	4.103	1.987	UNIT WEIGHT(LBS/FT ³)	163.6





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Method C



DM

PROJECT NO:	20050114COL	UNIAXIAL COMPRESSIVE STRENGTH OF	
DATE:	9/3/2020	INTACT ROCK CORE - ASTM D 7012	



Method C

BORING NUMBER	B-004-2-20	TOP DEPTH(FT)	42.4	BOTTOM DEPTH(FT)	43.0
SAMPLE NUMBER	R-3	DISTRICT	10	PID NO.	108676
COUNTY	MOE	ROUTE	7	SECTION	7.55 (10-K)

FORMATION	Pennsylvanian Age
DESCRIPTION	Sandstone, Gray, Slightly Weathered, Strong
MOISTURE CONDITION	As Received

			-	
MEASUREMENT	LENGTH(INCHES)	DIAMETER(INCHES)		LEN
1	4.091	1.990		CORR
2	4.096	1.991		
3	4.093	1.990		
AVERAGE	4.093	1.990	1	UNIT V

DATE:

LENGTH/DIAMETER	2.1
CORRECTION FACTOR	1
AREA(IN ²)	3.1
MASS (GRAMS)	540.4
UNIT WEIGHT(LBS/FT ³)	161.6





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DRAWN CLW	CHECKED
SOIL PROFILE - LANDSLIDE	COMPRESSIVE STRENGTH TEST RESULTS
	MOE - 1 - 1.00
34	734